



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	115 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 20.76 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

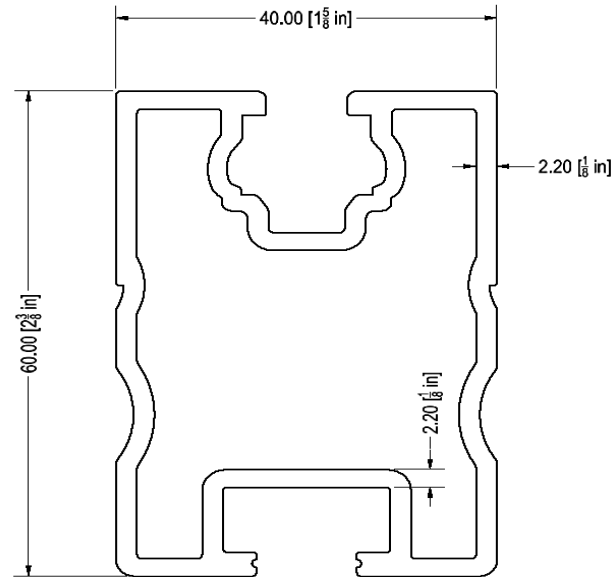
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	81 in
ΦF_{ty} STRONG-AXIS =	28.63 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.668 k-ft
M_z =	0.184 k-ft
$M_{y \text{ allowable}}$ =	1.218 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	76%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.46 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.640 k-ft
M_z =	0.000 k-ft
P_n =	0.295 k
$M_{y \text{ allowable}}$ =	1.446 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	47%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.047 k-ft
P_n =	0.222 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	13%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.787 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	21%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.811 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.235 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	15%



A cross brace kit is required every 13 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

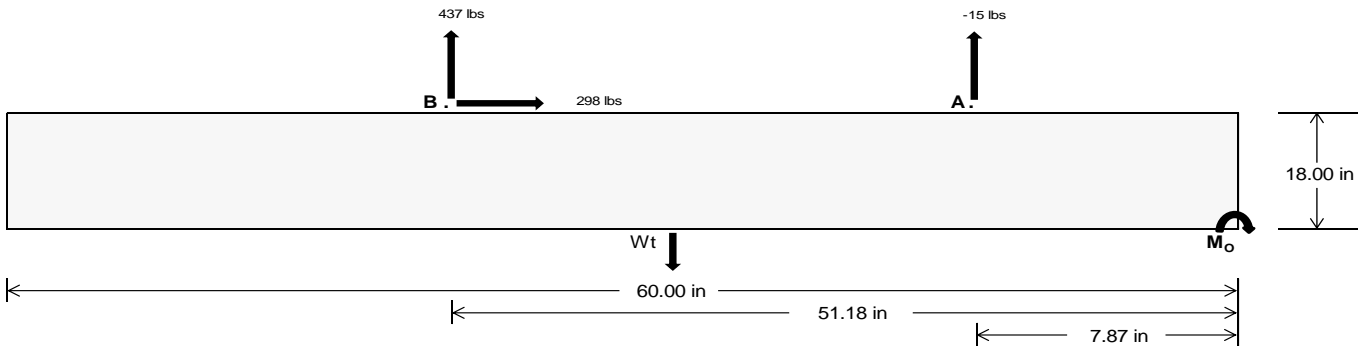
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	17.09	1900.04	k
Compressive Load =	1229.85	1368.90	k
Lateral Load =	38.29	1291.70	k
Moment (Weak Axis) =	0.06	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27626.0$ in-lbs
Resisting Force Required = 920.87 lbs
S.F. = 1.67
Weight Required = 1534.78 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 297.85 lbs
Friction = 0.4
Weight Required = 744.63 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 297.85 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	480 lbs	480 lbs	480 lbs	480 lbs	356 lbs	356 lbs	356 lbs	356 lbs	579 lbs	579 lbs	579 lbs	579 lbs	30 lbs	30 lbs	30 lbs	30 lbs
F_B	323 lbs	323 lbs	323 lbs	323 lbs	573 lbs	573 lbs	573 lbs	573 lbs	639 lbs	639 lbs	639 lbs	639 lbs	-875 lbs	-875 lbs	-875 lbs	-875 lbs
F_V	62 lbs	62 lbs	62 lbs	62 lbs	545 lbs	545 lbs	545 lbs	545 lbs	450 lbs	450 lbs	450 lbs	450 lbs	-596 lbs	-596 lbs	-596 lbs	-596 lbs
P_{total}	2707 lbs	2797 lbs	2888 lbs	2979 lbs	2833 lbs	2924 lbs	3014 lbs	3105 lbs	3121 lbs	3212 lbs	3303 lbs	3393 lbs	297 lbs	352 lbs	406 lbs	460 lbs
M	408 lbs-ft	408 lbs-ft	408 lbs-ft	408 lbs-ft	463 lbs-ft	463 lbs-ft	463 lbs-ft	463 lbs-ft	616 lbs-ft	616 lbs-ft	616 lbs-ft	616 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft
e	0.15 ft	0.15 ft	0.14 ft	0.14 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.37 ft	2.01 ft	1.74 ft	1.53 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	253.3 psf	251.7 psf	250.2 psf	248.9 psf	260.2 psf	258.3 psf	256.5 psf	254.9 psf	272.3 psf	269.8 psf	267.5 psf	265.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	365.3 psf	358.6 psf	352.5 psf	346.8 psf	387.3 psf	379.6 psf	372.5 psf	366.1 psf	441.2 psf	431.0 psf	421.7 psf	413.2 psf	898.7 psf	259.3 psf	185.4 psf	158.7 psf

Maximum Bearing Pressure = 899 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

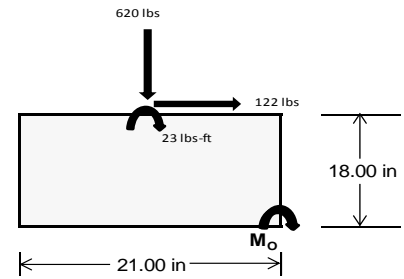
Overturning Check

$M_o = 335.8 \text{ ft-lbs}$
 Resisting Force Required = 383.74 lbs
 S.F. = 1.67
 Weight Required = 639.56 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	145 lbs	132 lbs	82 lbs	293 lbs	620 lbs	245 lbs	88 lbs	-11 lbs	28 lbs
F_v	20 lbs	162 lbs	21 lbs	14 lbs	122 lbs	16 lbs	21 lbs	162 lbs	21 lbs
P_{total}	2502 lbs	2488 lbs	2438 lbs	2536 lbs	2863 lbs	2487 lbs	777 lbs	678 lbs	717 lbs
M	59 lbs-ft	275 lbs-ft	63 lbs-ft	39 lbs-ft	207 lbs-ft	50 lbs-ft	60 lbs-ft	274 lbs-ft	63 lbs-ft
e	0.02 ft	0.11 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.08 ft	0.40 ft	0.09 ft
$L/6$	0.29 ft	1.53 ft	1.70 ft	1.72 ft	1.61 ft	1.71 ft	1.59 ft	0.94 ft	1.58 ft
f_{min}	262.9 sqft	176.7 sqft	254.1 sqft	274.8 sqft	246.2 sqft	264.8 sqft	65.2 sqft	-30.0 sqft	57.4 sqft
f_{max}	308.9 psf	392.0 psf	303.2 psf	304.9 psf	408.1 psf	303.7 psf	112.4 psf	185.0 psf	106.4 psf



Maximum Bearing Pressure = 408 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.464 k
Allowable Uplift =	1.214 k
Utilization =	<u>38%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.112 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.946 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Diagonal Strut

Maximum Axial Load =	0.787 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.189 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.235 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.099 in
	<u>0.099 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$210.919$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$219.027$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.218 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1371 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.446 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMMini 60 Cell 1V 35° 115mph 30psf 6.75ft 7-10Pa Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	252.159	1	-.031	15	.432	1	0	10	.001	4	0	15
30			min	-366.477	3	-.129	4	-.432	5	0	1	0	3	0	6
31		16	max	252.294	1	-.044	15	.432	1	0	10	.001	4	0	15
32			min	-366.376	3	-.186	4	-.555	5	0	1	0	3	0	6
33		17	max	252.428	1	-.058	15	.432	1	0	10	.001	4	0	15
34			min	-366.274	3	-.244	4	-.678	5	0	1	0	3	0	6
35		18	max	252.563	1	-.071	15	.432	1	0	10	.001	1	0	15
36			min	-366.173	3	-.301	4	-.801	5	0	1	0	3	0	6
37		19	max	252.698	1	-.085	15	.432	1	0	10	.001	1	0	15
38			min	-366.072	3	-.359	4	-.924	5	0	1	0	3	0	6
39	M3	1	max	205.435	2	1.734	6	-.035	12	0	5	.002	1	0	6
40			min	-217.878	3	.407	15	-1.399	4	0	1	0	12	0	15
41		2	max	205.365	2	1.557	6	-.035	12	0	5	.002	1	0	2
42			min	-217.931	3	.366	15	-1.265	4	0	1	0	12	0	3
43		3	max	205.295	2	1.381	6	-.035	12	0	5	.001	1	0	2
44			min	-217.983	3	.324	15	-1.132	4	0	1	0	15	0	3
45		4	max	205.225	2	1.204	6	-.035	12	0	5	.001	1	0	15
46			min	-218.036	3	.283	15	-.998	4	0	1	0	5	0	4
47		5	max	205.155	2	1.028	6	-.035	12	0	5	.001	1	0	15
48			min	-218.088	3	.241	15	-.864	4	0	1	0	5	0	4
49		6	max	205.085	2	.852	6	-.035	12	0	5	.001	1	0	15
50			min	-218.141	3	.2	15	-.731	4	0	1	0	5	0	4
51		7	max	205.015	2	.675	6	-.035	12	0	5	.001	1	0	15
52			min	-218.193	3	.158	15	-.597	4	0	1	0	5	0	4
53		8	max	204.945	2	.499	6	-.035	12	0	5	0	1	0	15
54			min	-218.246	3	.117	15	-.484	1	0	1	0	5	-.001	4
55		9	max	204.875	2	.323	6	-.035	12	0	5	0	1	0	15
56			min	-218.298	3	.075	15	-.484	1	0	1	0	5	-.001	4
57		10	max	204.805	2	.146	6	-.035	12	0	5	0	1	0	15
58			min	-218.351	3	.034	15	-.484	1	0	1	0	5	-.001	4
59		11	max	204.735	2	.004	2	.049	5	0	5	0	1	0	15
60			min	-218.403	3	-.054	3	-.484	1	0	1	0	5	-.001	4
61		12	max	204.665	2	-.049	15	.182	5	0	5	0	1	0	15
62			min	-218.456	3	-.207	4	-.484	1	0	1	0	5	-.001	4
63		13	max	204.595	2	-.09	15	.316	5	0	5	0	1	0	15
64			min	-218.508	3	-.383	4	-.484	1	0	1	0	5	-.001	4
65		14	max	204.525	2	-.132	15	.45	5	0	5	0	1	0	15
66			min	-218.561	3	-.559	4	-.484	1	0	1	0	5	-.001	4
67		15	max	204.455	2	-.173	15	.583	5	0	5	0	1	0	15
68			min	-218.613	3	-.736	4	-.484	1	0	1	0	5	0	4
69		16	max	204.385	2	-.215	15	.717	5	0	5	0	1	0	15
70			min	-218.666	3	-.912	4	-.484	1	0	1	0	5	0	4
71		17	max	204.315	2	-.256	15	.851	5	0	5	0	12	0	15
72			min	-218.718	3	-1.088	4	-.484	1	0	1	0	5	0	4
73		18	max	204.245	2	-.298	15	.984	5	0	5	0	12	0	15
74			min	-218.771	3	-1.265	4	-.484	1	0	1	0	1	0	4
75		19	max	204.175	2	-.339	15	1.118	5	0	5	0	5	0	1
76			min	-218.823	3	-1.441	4	-.484	1	0	1	0	1	0	1
77	M4	1	max	376.731	1	0	1	-.212	10	0	1	0	5	0	1
78			min	24.564	15	0	1	-28.73	4	0	1	0	2	0	1
79		2	max	376.796	1	0	1	-.212	10	0	1	0	12	0	1
80			min	24.583	15	0	1	-28.786	4	0	1	-.003	4	0	1
81		3	max	376.86	1	0	1	-.212	10	0	1	0	12	0	1
82			min	24.603	15	0	1	-28.842	4	0	1	-.005	4	0	1
83		4	max	376.925	1	0	1	-.212	10	0	1	0	12	0	1
84			min	24.622	15	0	1	-28.898	4	0	1	-.008	4	0	1
85		5	max	376.99	1	0	1	-.212	10	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	24.642	15	0	1	-28.954	4	0	1	-.01	4	0	1
87		6	max	377.054	1	0	1	-.212	10	0	1	0	12	0	1
88			min	24.661	15	0	1	-29.01	4	0	1	-.013	4	0	1
89		7	max	377.119	1	0	1	-.212	10	0	1	0	12	0	1
90			min	24.681	15	0	1	-29.066	4	0	1	-.015	4	0	1
91		8	max	377.184	1	0	1	-.212	10	0	1	0	12	0	1
92			min	24.7	15	0	1	-29.122	4	0	1	-.018	4	0	1
93		9	max	377.248	1	0	1	-.212	10	0	1	0	12	0	1
94			min	24.72	15	0	1	-29.178	4	0	1	-.021	4	0	1
95		10	max	377.313	1	0	1	-.212	10	0	1	0	12	0	1
96			min	24.739	15	0	1	-29.234	4	0	1	-.023	4	0	1
97		11	max	377.378	1	0	1	-.212	10	0	1	0	12	0	1
98			min	24.759	15	0	1	-29.291	4	0	1	-.026	4	0	1
99		12	max	377.443	1	0	1	-.212	10	0	1	0	12	0	1
100			min	24.778	15	0	1	-29.347	4	0	1	-.029	4	0	1
101		13	max	377.507	1	0	1	-.212	10	0	1	0	12	0	1
102			min	24.798	15	0	1	-29.403	4	0	1	-.031	4	0	1
103		14	max	377.572	1	0	1	-.212	10	0	1	0	12	0	1
104			min	24.817	15	0	1	-29.459	4	0	1	-.034	4	0	1
105		15	max	377.637	1	0	1	-.212	10	0	1	0	12	0	1
106			min	24.837	15	0	1	-29.515	4	0	1	-.036	4	0	1
107		16	max	377.701	1	0	1	-.212	10	0	1	0	12	0	1
108			min	24.856	15	0	1	-29.571	4	0	1	-.039	4	0	1
109		17	max	377.766	1	0	1	-.212	10	0	1	0	12	0	1
110			min	24.876	15	0	1	-29.627	4	0	1	-.042	4	0	1
111		18	max	377.831	1	0	1	-.212	10	0	1	0	12	0	1
112			min	24.895	15	0	1	-29.683	4	0	1	-.044	4	0	1
113		19	max	377.896	1	0	1	-.212	10	0	1	0	12	0	1
114			min	24.915	15	0	1	-29.739	4	0	1	-.047	4	0	1
115	M6	1	max	808.612	1	.664	6	1.274	4	0	3	0	3	0	1
116			min	-1188.565	3	.148	15	-.163	3	0	5	0	1	0	1
117		2	max	808.747	1	.607	6	1.151	4	0	3	0	4	0	15
118			min	-1188.464	3	.134	15	-.163	3	0	5	0	11	0	6
119		3	max	808.882	1	.549	6	1.027	4	0	3	0	4	0	15
120			min	-1188.363	3	.121	15	-.163	3	0	5	0	11	0	6
121		4	max	809.017	1	.493	2	.904	4	0	3	0	4	0	15
122			min	-1188.262	3	.107	15	-.163	3	0	5	0	10	0	6
123		5	max	809.152	1	.448	2	.781	4	0	3	0	4	0	15
124			min	-1188.161	3	.094	15	-.163	3	0	5	0	10	0	6
125		6	max	809.287	1	.403	2	.658	4	0	3	.001	4	0	15
126			min	-1188.06	3	.076	12	-.163	3	0	5	0	10	0	6
127		7	max	809.422	1	.359	2	.535	4	0	3	.001	4	0	15
128			min	-1187.959	3	.054	12	-.163	3	0	5	0	10	0	2
129		8	max	809.556	1	.314	2	.412	4	0	3	.001	4	0	15
130			min	-1187.857	3	.032	12	-.163	3	0	5	0	3	0	2
131		9	max	809.691	1	.269	2	.288	4	0	3	.001	4	0	15
132			min	-1187.756	3	.001	3	-.163	3	0	5	0	3	0	2
133		10	max	809.826	1	.224	2	.165	4	0	3	.001	4	0	15
134			min	-1187.655	3	-.032	3	-.163	3	0	5	0	3	0	2
135		11	max	809.961	1	.179	2	.137	1	0	3	.001	4	0	12
136			min	-1187.554	3	-.066	3	-.163	3	0	5	0	3	0	2
137		12	max	810.096	1	.135	2	.137	1	0	3	.001	4	0	12
138			min	-1187.453	3	-.1	3	-.163	3	0	5	0	3	0	2
139		13	max	810.231	1	.09	2	.137	1	0	3	.001	4	0	12
140			min	-1187.352	3	-.133	3	-.244	5	0	5	0	3	0	2
141		14	max	810.366	1	.045	2	.137	1	0	3	.001	4	0	12
142			min	-1187.251	3	-.167	3	-.368	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	810.501	1	0	2	.137	1	0	3	.001	4	0	12
144		min	-1187.149	3	-.2	3	-.491	5	0	5	0	3	0	2
145	16	max	810.635	1	-.045	2	.137	1	0	3	.001	4	0	3
146		min	-1187.048	3	-.234	3	-.614	5	0	5	0	3	0	2
147	17	max	810.77	1	-.069	15	.137	1	0	3	.001	4	0	3
148		min	-1186.947	3	-.268	3	-.737	5	0	5	0	3	0	2
149	18	max	810.905	1	-.082	15	.137	1	0	3	0	4	0	3
150		min	-1186.846	3	-.314	4	-.86	5	0	5	0	3	0	2
151	19	max	811.04	1	-.096	15	.137	1	0	3	0	4	0	3
152		min	-1186.745	3	-.371	4	-.983	5	0	5	0	3	0	2
153	M7	1	max	787.071	2	1.756	.03	3	0	14	0	4	0	2
154		min	-682.521	3	.42	15	-1.291	5	0	3	0	3	0	3
155	2	max	787.001	2	1.58	4	.03	3	0	14	0	4	0	2
156		min	-682.573	3	.378	15	-1.157	5	0	3	0	3	0	3
157	3	max	786.931	2	1.403	4	.03	3	0	14	0	2	0	2
158		min	-682.626	3	.337	15	-1.023	5	0	3	0	3	0	3
159	4	max	786.861	2	1.227	4	.03	3	0	14	0	2	0	2
160		min	-682.678	3	.295	15	-.89	5	0	3	0	5	0	3
161	5	max	786.791	2	1.051	4	.03	3	0	14	0	2	0	15
162		min	-682.731	3	.254	15	-.756	5	0	3	0	5	0	3
163	6	max	786.721	2	.874	4	.03	3	0	14	0	2	0	15
164		min	-682.783	3	.212	15	-.622	5	0	3	0	5	0	3
165	7	max	786.651	2	.698	4	.03	3	0	14	0	2	0	15
166		min	-682.836	3	.171	15	-.489	5	0	3	0	5	0	6
167	8	max	786.581	2	.521	4	.03	3	0	14	0	2	0	15
168		min	-682.888	3	.122	12	-.355	5	0	3	0	5	-.001	6
169	9	max	786.511	2	.349	2	.03	3	0	14	0	2	0	15
170		min	-682.941	3	.054	12	-.221	5	0	3	0	5	-.001	6
171	10	max	786.441	2	.212	2	.03	3	0	14	0	2	0	15
172		min	-682.993	3	-.035	3	-.088	5	0	3	0	5	-.001	6
173	11	max	786.371	2	.075	2	.047	4	0	14	0	2	0	15
174		min	-683.046	3	-.138	3	-.008	2	0	3	0	5	-.001	6
175	12	max	786.301	2	-.036	15	.181	4	0	14	0	2	0	15
176		min	-683.098	3	-.241	3	-.008	2	0	3	0	5	-.001	6
177	13	max	786.231	2	-.078	15	.315	4	0	14	0	2	0	15
178		min	-683.151	3	-.361	6	-.008	2	0	3	0	5	-.001	6
179	14	max	786.161	2	-.119	15	.448	4	0	14	0	11	0	15
180		min	-683.203	3	-.537	6	-.008	2	0	3	0	5	-.001	6
181	15	max	786.091	2	-.161	15	.582	4	0	14	0	11	0	15
182		min	-683.256	3	-.714	6	-.008	2	0	3	0	5	0	6
183	16	max	786.021	2	-.202	15	.716	4	0	14	0	11	0	15
184		min	-683.308	3	-.89	6	-.008	2	0	3	0	5	0	6
185	17	max	785.951	2	-.244	15	.849	4	0	14	0	11	0	15
186		min	-683.361	3	-1.067	6	-.008	2	0	3	0	5	0	6
187	18	max	785.881	2	-.285	15	.983	4	0	14	0	1	0	15
188		min	-683.413	3	-1.243	6	-.008	2	0	3	0	3	0	6
189	19	max	785.811	2	-.327	15	1.117	4	0	14	0	14	0	1
190		min	-683.466	3	-1.419	6	-.008	2	0	3	0	3	0	1
191	M8	1	max	944.874	1	0	.771	1	0	1	0	4	0	1
192		min	32.9	15	0	1	-28.749	4	0	1	0	1	0	1
193	2	max	944.938	1	0	1	.771	1	0	1	0	1	0	1
194		min	32.919	15	0	1	-28.805	4	0	1	-.003	4	0	1
195	3	max	945.003	1	0	1	.771	1	0	1	0	1	0	1
196		min	32.939	15	0	1	-28.861	4	0	1	-.005	4	0	1
197	4	max	945.068	1	0	1	.771	1	0	1	0	1	0	1
198		min	32.958	15	0	1	-28.917	4	0	1	-.008	4	0	1
199	5	max	945.133	1	0	1	.771	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	32.978	15	0	1	-28.974	4	0	1	-.01	4	0	1
201		6	max	945.197	1	0	1	.771	1	0	1	0	1	0	1
202			min	32.997	15	0	1	-29.03	4	0	1	-.013	4	0	1
203		7	max	945.262	1	0	1	.771	1	0	1	0	1	0	1
204			min	33.017	15	0	1	-29.086	4	0	1	-.015	4	0	1
205		8	max	945.327	1	0	1	.771	1	0	1	0	1	0	1
206			min	33.036	15	0	1	-29.142	4	0	1	-.018	4	0	1
207		9	max	945.391	1	0	1	.771	1	0	1	0	1	0	1
208			min	33.056	15	0	1	-29.198	4	0	1	-.021	4	0	1
209		10	max	945.456	1	0	1	.771	1	0	1	0	1	0	1
210			min	33.075	15	0	1	-29.254	4	0	1	-.023	4	0	1
211		11	max	945.521	1	0	1	.771	1	0	1	0	1	0	1
212			min	33.095	15	0	1	-29.31	4	0	1	-.026	4	0	1
213		12	max	945.585	1	0	1	.771	1	0	1	0	1	0	1
214			min	33.114	15	0	1	-29.366	4	0	1	-.029	4	0	1
215		13	max	945.65	1	0	1	.771	1	0	1	0	1	0	1
216			min	33.134	15	0	1	-29.422	4	0	1	-.031	4	0	1
217		14	max	945.715	1	0	1	.771	1	0	1	0	1	0	1
218			min	33.153	15	0	1	-29.478	4	0	1	-.034	4	0	1
219		15	max	945.78	1	0	1	.771	1	0	1	0	1	0	1
220			min	33.173	15	0	1	-29.534	4	0	1	-.036	4	0	1
221		16	max	945.844	1	0	1	.771	1	0	1	.001	1	0	1
222			min	33.192	15	0	1	-29.59	4	0	1	-.039	4	0	1
223		17	max	945.909	1	0	1	.771	1	0	1	.001	1	0	1
224			min	33.212	15	0	1	-29.646	4	0	1	-.042	4	0	1
225		18	max	945.974	1	0	1	.771	1	0	1	.001	1	0	1
226			min	33.231	15	0	1	-29.703	4	0	1	-.044	4	0	1
227		19	max	946.038	1	0	1	.771	1	0	1	.001	1	0	1
228			min	33.251	15	0	1	-29.759	4	0	1	-.047	4	0	1
229	M10	1	max	259.781	1	.707	4	1.442	5	0	1	0	1	0	1
230			min	-336.233	3	.179	15	-.185	1	-.002	5	0	5	0	1
231		2	max	259.916	1	.65	4	1.319	5	0	1	0	1	0	15
232			min	-336.132	3	.166	15	-.185	1	-.002	5	0	3	0	4
233		3	max	260.051	1	.592	4	1.196	5	0	1	0	4	0	15
234			min	-336.031	3	.152	15	-.185	1	-.002	5	0	3	0	4
235		4	max	260.186	1	.535	4	1.072	5	0	1	0	4	0	15
236			min	-335.929	3	.139	15	-.185	1	-.002	5	0	3	0	4
237		5	max	260.321	1	.477	4	.949	5	0	1	0	4	0	15
238			min	-335.828	3	.125	15	-.185	1	-.002	5	0	3	0	4
239		6	max	260.456	1	.42	4	.826	5	0	1	0	4	0	15
240			min	-335.727	3	.112	15	-.185	1	-.002	5	0	3	0	4
241		7	max	260.591	1	.362	4	.703	5	0	1	.001	4	0	15
242			min	-335.626	3	.098	15	-.185	1	-.002	5	0	3	0	4
243		8	max	260.725	1	.305	4	.58	5	0	1	.001	4	0	15
244			min	-335.525	3	.085	15	-.185	1	-.002	5	0	3	0	4
245		9	max	260.86	1	.247	4	.457	5	0	1	.001	4	0	15
246			min	-335.424	3	.071	15	-.185	1	-.002	5	0	3	0	4
247		10	max	260.995	1	.19	4	.333	5	0	1	.001	4	0	15
248			min	-335.323	3	.052	12	-.185	1	-.002	5	0	3	0	4
249		11	max	261.13	1	.132	4	.21	5	0	1	.001	4	0	15
250			min	-335.221	3	.03	12	-.185	1	-.002	5	0	3	0	4
251		12	max	261.265	1	.075	4	.087	5	0	1	.001	4	0	15
252			min	-335.12	3	.008	12	-.185	1	-.002	5	0	3	0	4
253		13	max	261.4	1	.025	5	.006	3	0	1	.001	4	0	15
254			min	-335.019	3	-.024	3	-.185	1	-.002	5	0	3	0	4
255		14	max	261.535	1	.004	5	.006	3	0	1	.001	4	0	15
256			min	-334.918	3	-.066	1	-.193	4	-.002	5	0	3	0	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	261.669	1	-.01	15	.006	3	0	1	.001	5	0	15
258		min	-334.817	3	-.11	1	-.317	4	-.002	5	0	3	0	4
259	16	max	261.804	1	-.023	15	.006	3	0	1	.001	5	0	15
260		min	-334.716	3	-.156	6	-.44	4	-.002	5	0	1	0	4
261	17	max	261.939	1	-.037	15	.006	3	0	1	.001	5	0	15
262		min	-334.614	3	-.214	6	-.563	4	-.002	5	0	1	0	4
263	18	max	262.074	1	-.05	15	.006	3	0	1	.001	5	0	12
264		min	-334.513	3	-.271	6	-.686	4	-.002	5	0	1	0	4
265	19	max	262.209	1	-.064	15	.006	3	0	1	0	5	0	12
266		min	-334.412	3	-.329	6	-.809	4	-.002	5	0	1	0	4
267	M11	1	max	205.12	2	1.721	.549	1	.002	4	0	5	0	1
268		min	-218.539	3	.398	15	-1.216	5	0	10	-.002	1	0	15
269	2	max	205.05	2	1.545	.549	1	.002	4	0	5	0	1	
270		min	-218.592	3	.357	15	-1.082	5	0	10	-.002	1	0	3
271	3	max	204.98	2	1.368	.549	1	.002	4	0	5	0	1	
272		min	-218.644	3	.315	15	-.949	5	0	10	-.001	1	0	3
273	4	max	204.91	2	1.192	.549	1	.002	4	0	3	0	15	
274		min	-218.697	3	.274	15	-.815	5	0	10	-.001	1	0	4
275	5	max	204.84	2	1.015	.549	1	.002	4	0	3	0	15	
276		min	-218.749	3	.232	15	-.681	5	0	10	-.001	1	0	4
277	6	max	204.77	2	.839	.549	1	.002	4	0	3	0	15	
278		min	-218.802	3	.191	15	-.548	5	0	10	-.001	1	0	4
279	7	max	204.7	2	.663	.549	1	.002	4	0	3	0	15	
280		min	-218.854	3	.149	15	-.414	5	0	10	-.001	1	-.001	4
281	8	max	204.63	2	.486	.549	1	.002	4	0	3	0	15	
282		min	-218.907	3	.108	15	-.28	5	0	10	0	1	-.001	4
283	9	max	204.56	2	.31	.549	1	.002	4	0	3	0	15	
284		min	-218.959	3	.066	15	-.147	5	0	10	0	1	-.001	4
285	10	max	204.49	2	.143	.549	1	.002	4	0	3	0	15	
286		min	-219.012	3	.025	15	-.02	3	0	10	0	1	-.001	4
287	11	max	204.42	2	.005	.549	1	.002	4	0	3	0	15	
288		min	-219.064	3	-.071	3	-.02	3	0	10	0	1	-.001	4
289	12	max	204.35	2	-.058	.549	1	.002	4	0	3	0	15	
290		min	-219.117	3	-.22	4	-.02	3	0	10	0	1	-.001	4
291	13	max	204.28	2	-.099	.549	1	.002	4	0	3	0	15	
292		min	-219.169	3	-.396	4	-.02	3	0	10	0	1	-.001	4
293	14	max	204.21	2	-.141	.643	4	.002	4	0	3	0	15	
294		min	-219.222	3	-.572	4	-.02	3	0	10	0	1	-.001	4
295	15	max	204.14	2	-.182	.777	4	.002	4	0	3	0	15	
296		min	-219.274	3	-.749	4	-.02	3	0	10	0	1	0	4
297	16	max	204.07	2	-.224	.91	4	.002	4	0	3	0	15	
298		min	-219.327	3	-.925	4	-.02	3	0	10	0	10	0	4
299	17	max	204	2	-.265	1.044	4	.002	4	0	4	0	15	
300		min	-219.379	3	-1.102	4	-.02	3	0	10	0	10	0	4
301	18	max	203.93	2	-.307	1.178	4	.002	4	0	4	0	15	
302		min	-219.432	3	-1.278	4	-.02	3	0	10	0	10	0	4
303	19	max	203.86	2	-.348	1.311	4	.002	4	0	4	0	1	
304		min	-219.484	3	-1.454	4	-.02	3	0	10	0	10	0	1
305	M12	1	max	376.387	1	0	3.415	1	0	1	0	4	0	1
306		min	7.853	15	0	1	-26.303	5	0	1	0	3	0	1
307	2	max	376.452	1	0	1	3.415	1	0	1	0	1	0	1
308		min	7.872	15	0	1	-26.359	5	0	1	-.002	5	0	1
309	3	max	376.517	1	0	1	3.415	1	0	1	0	1	0	1
310		min	7.892	15	0	1	-26.415	5	0	1	-.005	5	0	1
311	4	max	376.581	1	0	1	3.415	1	0	1	0	1	0	1
312		min	7.911	15	0	1	-26.471	5	0	1	-.007	5	0	1
313	5	max	376.646	1	0	1	3.415	1	0	1	.001	1	0	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	7.931	15	0	1	-26.527	5	0	1	-.009	5	0	1
315		6	max	376.711	1	0	1	3.415	1	0	1	.002	1	0	1
316			min	7.95	15	0	1	-26.583	5	0	1	-.012	5	0	1
317		7	max	376.775	1	0	1	3.415	1	0	1	.002	1	0	1
318			min	7.97	15	0	1	-26.639	5	0	1	-.014	5	0	1
319		8	max	376.84	1	0	1	3.415	1	0	1	.002	1	0	1
320			min	7.989	15	0	1	-26.695	5	0	1	-.017	5	0	1
321		9	max	376.905	1	0	1	3.415	1	0	1	.002	1	0	1
322			min	8.009	15	0	1	-26.751	5	0	1	-.019	5	0	1
323		10	max	376.969	1	0	1	3.415	1	0	1	.003	1	0	1
324			min	8.029	15	0	1	-26.807	5	0	1	-.021	5	0	1
325		11	max	377.034	1	0	1	3.415	1	0	1	.003	1	0	1
326			min	8.048	15	0	1	-26.864	5	0	1	-.024	5	0	1
327		12	max	377.099	1	0	1	3.415	1	0	1	.003	1	0	1
328			min	8.068	15	0	1	-26.92	5	0	1	-.026	5	0	1
329		13	max	377.164	1	0	1	3.415	1	0	1	.004	1	0	1
330			min	8.087	15	0	1	-26.976	5	0	1	-.029	5	0	1
331		14	max	377.228	1	0	1	3.415	1	0	1	.004	1	0	1
332			min	8.107	15	0	1	-27.032	5	0	1	-.031	5	0	1
333		15	max	377.293	1	0	1	3.415	1	0	1	.004	1	0	1
334			min	8.126	15	0	1	-27.088	5	0	1	-.033	5	0	1
335		16	max	377.358	1	0	1	3.415	1	0	1	.005	1	0	1
336			min	8.146	15	0	1	-27.144	5	0	1	-.036	5	0	1
337		17	max	377.422	1	0	1	3.415	1	0	1	.005	1	0	1
338			min	8.165	15	0	1	-27.2	5	0	1	-.038	5	0	1
339		18	max	377.487	1	0	1	3.415	1	0	1	.005	1	0	1
340			min	8.185	15	0	1	-27.256	5	0	1	-.041	5	0	1
341		19	max	377.552	1	0	1	3.415	1	0	1	.006	1	0	1
342			min	8.204	15	0	1	-27.312	5	0	1	-.043	5	0	1
343	M1	1	max	139.552	1	343.696	3	-4.092	12	0	1	.134	1	0	2
344			min	7.324	12	-247.179	1	-67.78	1	0	3	.009	12	0	3
345		2	max	139.712	1	343.524	3	-4.092	12	0	1	.119	1	.054	1
346			min	7.404	12	-247.408	1	-67.78	1	0	3	.008	12	-.075	3
347		3	max	116.94	3	6.523	9	-4.115	12	0	12	.103	1	.107	1
348			min	-15.095	10	-28.599	2	-67.693	1	0	1	.007	12	-.148	3
349		4	max	117.06	3	6.332	9	-4.115	12	0	12	.089	1	.111	2
350			min	-14.961	10	-28.828	2	-67.693	1	0	1	.006	12	-.146	3
351		5	max	117.18	3	6.142	9	-4.115	12	0	12	.074	1	.117	2
352			min	-14.828	10	-29.057	2	-67.693	1	0	1	.005	12	-.144	3
353		6	max	117.3	3	5.951	9	-4.115	12	0	12	.059	1	.124	2
354			min	-14.694	10	-29.286	2	-67.693	1	0	1	.004	12	-.142	3
355		7	max	117.42	3	5.76	9	-4.115	12	0	12	.044	1	.13	2
356			min	-14.561	10	-29.514	2	-67.693	1	0	1	.003	12	-.14	3
357		8	max	117.54	3	5.57	9	-4.115	12	0	12	.03	1	.137	2
358			min	-14.427	10	-29.743	2	-67.693	1	0	1	.003	12	-.137	3
359		9	max	117.66	3	5.379	9	-4.115	12	0	12	.015	1	.143	2
360			min	-14.294	10	-29.972	2	-67.693	1	0	1	.002	10	-.135	3
361		10	max	117.781	3	5.189	9	-4.115	12	0	12	.002	4	.15	2
362			min	-14.16	10	-30.201	2	-67.693	1	0	1	0	10	-.133	3
363		11	max	117.901	3	4.998	9	-4.115	12	0	12	0	3	.156	2
364			min	-14.027	10	-30.429	2	-67.693	1	0	1	-.014	1	-.131	3
365		12	max	118.021	3	4.807	9	-4.115	12	0	12	-.001	12	.163	2
366			min	-13.893	10	-30.658	2	-67.693	1	0	1	-.029	1	-.129	3
367		13	max	118.141	3	4.617	9	-4.115	12	0	12	-.002	12	.169	2
368			min	-13.76	10	-30.887	2	-67.693	1	0	1	-.044	1	-.126	3
369		14	max	118.261	3	4.426	9	-4.115	12	0	12	-.003	12	.176	2
370			min	-13.627	10	-31.115	2	-67.693	1	0	1	-.058	1	-.124	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	118.381	3	4.236	9	-4.115	12	0	12	-.004	12	.183	2
372			min	-13.493	10	-31.344	2	-67.693	1	0	1	-.073	1	-.121	3
373		16	max	92.421	2	148.547	2	-4.152	12	0	1	-.005	12	.188	2
374			min	2.765	15	-204.991	3	-68.088	1	0	5	-.088	1	-.117	3
375		17	max	92.581	2	148.318	2	-4.152	12	0	1	-.006	12	.156	2
376			min	2.813	15	-205.163	3	-68.088	1	0	5	-.103	1	-.073	3
377		18	max	-6.563	12	361.084	2	-4.372	12	0	3	-.006	12	.079	2
378			min	-139.403	1	-167.186	3	-69.819	1	0	2	-.118	1	-.036	3
379		19	max	-6.483	12	360.855	2	-4.372	12	0	3	-.007	12	0	2
380			min	-139.243	1	-167.358	3	-69.819	1	0	2	-.133	1	0	3
381	M5	1	max	307.906	1	1134.094	3	-.036	10	0	1	.04	4	0	3
382			min	10.891	15	-816.354	1	-52.155	3	0	5	0	10	0	2
383		2	max	308.066	1	1133.923	3	-.036	10	0	1	.035	4	.177	1
384			min	10.94	15	-816.583	1	-52.155	3	0	5	-.006	3	-.246	3
385		3	max	365.846	3	5.51	9	5.992	3	0	3	.029	4	.351	1
386			min	-76.513	2	-105.42	2	-21.862	4	0	4	-.016	3	-.486	3
387		4	max	365.966	3	5.32	9	5.992	3	0	3	.024	4	.367	2
388			min	-76.353	2	-105.649	2	-21.62	4	0	4	-.015	3	-.479	3
389		5	max	366.087	3	5.129	9	5.992	3	0	3	.02	4	.39	2
390			min	-76.193	2	-105.877	2	-21.378	4	0	4	-.014	3	-.471	3
391		6	max	366.207	3	4.938	9	5.992	3	0	3	.015	4	.413	2
392			min	-76.033	2	-106.106	2	-21.136	4	0	4	-.012	3	-.463	3
393		7	max	366.327	3	4.748	9	5.992	3	0	3	.01	4	.436	2
394			min	-75.873	2	-106.335	2	-20.894	4	0	4	-.011	3	-.455	3
395		8	max	366.447	3	4.557	9	5.992	3	0	3	.006	4	.459	2
396			min	-75.713	2	-106.564	2	-20.652	4	0	4	-.01	3	-.447	3
397		9	max	366.567	3	4.367	9	5.992	3	0	3	.002	4	.483	2
398			min	-75.552	2	-106.792	2	-20.41	4	0	4	-.009	3	-.439	3
399		10	max	366.687	3	4.176	9	5.992	3	0	3	0	10	.506	2
400			min	-75.392	2	-107.021	2	-20.168	4	0	4	-.007	3	-.431	3
401		11	max	366.807	3	3.985	9	5.992	3	0	3	0	10	.529	2
402			min	-75.232	2	-107.25	2	-19.926	4	0	4	-.007	4	-.423	3
403		12	max	366.927	3	3.795	9	5.992	3	0	3	0	10	.552	2
404			min	-75.072	2	-107.478	2	-19.684	4	0	4	-.012	4	-.415	3
405		13	max	367.048	3	3.604	9	5.992	3	0	3	0	10	.576	2
406			min	-74.912	2	-107.707	2	-19.442	4	0	4	-.016	4	-.407	3
407		14	max	367.168	3	3.414	9	5.992	3	0	3	0	10	.599	2
408			min	-74.752	2	-107.936	2	-19.2	4	0	4	-.02	4	-.399	3
409		15	max	367.288	3	3.223	9	5.992	3	0	3	0	10	.622	2
410			min	-74.591	2	-108.165	2	-18.958	4	0	4	-.024	4	-.391	3
411		16	max	294.994	2	588.949	2	5.974	3	0	1	0	3	.64	2
412			min	3.4	15	-642.455	3	-17.613	4	0	4	-.028	4	-.377	3
413		17	max	295.154	2	588.72	2	5.974	3	0	1	.001	3	.512	2
414			min	3.449	15	-642.627	3	-17.371	4	0	4	-.032	4	-.238	3
415		18	max	-12.491	12	1187.333	2	5.449	3	0	4	.003	3	.257	2
416			min	-308.477	1	-547.928	3	-44.936	5	0	1	-.041	4	-.119	3
417		19	max	-12.411	12	1187.104	2	5.449	3	0	4	.004	3	0	3
418			min	-308.317	1	-548.1	3	-44.694	5	0	1	-.051	4	0	2
419	M9	1	max	138.946	1	343.655	3	192.178	4	0	3	-.003	15	0	2
420			min	4.367	15	-247.17	1	7.373	10	0	1	-.132	1	0	3
421		2	max	139.107	1	343.484	3	192.42	4	0	3	.034	5	.054	1
422			min	4.415	15	-247.399	1	7.373	10	0	1	-.115	1	-.075	3
423		3	max	117.087	3	6.503	9	64.965	1	0	1	.07	5	.106	1
424			min	-14.563	10	-28.607	2	-27.21	5	0	5	-.097	1	-.148	3
425		4	max	117.207	3	6.313	9	64.965	1	0	1	.064	5	.111	2
426			min	-14.429	10	-28.836	2	-26.968	5	0	5	-.083	1	-.146	3
427		5	max	117.327	3	6.122	9	64.965	1	0	1	.058	5	.117	2



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-14.296	10	-29.065	2	-26.726	5	0	5	-.069	1	-.144	3
429	6	max	117.447	3	5.931	9	64.965	1	0	1	.052	5	.124	2
430		min	-14.162	10	-29.294	2	-26.484	5	0	5	-.055	1	-.142	3
431	7	max	117.567	3	5.741	9	64.965	1	0	1	.047	5	.13	2
432		min	-14.029	10	-29.522	2	-26.242	5	0	5	-.04	1	-.14	3
433	8	max	117.687	3	5.55	9	64.965	1	0	1	.041	5	.137	2
434		min	-13.895	10	-29.751	2	-26	5	0	5	-.026	1	-.137	3
435	9	max	117.807	3	5.359	9	64.965	1	0	1	.035	5	.143	2
436		min	-13.762	10	-29.98	2	-25.758	5	0	5	-.012	1	-.135	3
437	10	max	117.928	3	5.169	9	64.965	1	0	1	.03	4	.15	2
438		min	-13.628	10	-30.209	2	-25.516	5	0	5	0	2	-.133	3
439	11	max	118.048	3	4.978	9	64.965	1	0	1	.028	4	.156	2
440		min	-13.495	10	-30.437	2	-25.274	5	0	5	.002	10	-.131	3
441	12	max	118.168	3	4.788	9	64.965	1	0	1	.03	1	.163	2
442		min	-13.361	10	-30.666	2	-25.032	5	0	5	.003	10	-.129	3
443	13	max	118.288	3	4.597	9	64.965	1	0	1	.044	1	.169	2
444		min	-13.228	10	-30.895	2	-24.79	5	0	5	.005	10	-.126	3
445	14	max	118.408	3	4.406	9	64.965	1	0	1	.058	1	.176	2
446		min	-13.094	10	-31.124	2	-24.548	5	0	5	.005	15	-.124	3
447	15	max	118.528	3	4.216	9	64.965	1	0	1	.072	1	.183	2
448		min	-12.961	10	-31.352	2	-24.306	5	0	5	.002	15	-.122	3
449	16	max	92.776	2	148.261	2	65.418	1	0	10	.087	1	.188	2
450		min	4.393	15	-205.504	3	-22.879	5	0	4	0	5	-.117	3
451	17	max	92.937	2	148.033	2	65.418	1	0	10	.101	1	.156	2
452		min	4.441	15	-205.675	3	-22.637	5	0	4	-.006	5	-.073	3
453	18	max	-.015	15	361.085	2	68.926	1	0	2	.116	1	.079	2
454		min	-139.004	1	-167.181	3	-48.622	5	0	3	-.016	5	-.036	3
455	19	max	.033	15	360.856	2	68.926	1	0	2	.131	1	0	2
456		min	-138.844	1	-167.353	3	-48.38	5	0	3	-.027	5	0	3
457	M13	1	max	192.193	4	246.786	1	-4.367	15	0	.132	1	0	1
458		min	7.376	10	-343.657	3	-138.93	1	0	3	.003	15	0	3
459	2	max	184.827	4	174.201	1	-2.896	15	0	2	.04	1	.22	3
460		min	7.376	10	-242.482	3	-106.352	1	0	3	0	15	-.158	1
461	3	max	177.461	4	101.617	1	-1.426	15	0	2	.004	3	.364	3
462		min	7.376	10	-141.307	3	-73.773	1	0	3	-.027	1	-.261	1
463	4	max	170.095	4	29.033	1	.045	15	0	2	0	3	.432	3
464		min	7.376	10	-40.132	3	-41.195	1	0	3	-.07	1	-.31	1
465	5	max	162.728	4	61.043	3	2.235	5	0	2	-.002	15	.424	3
466		min	7.376	10	-43.552	1	-8.617	1	0	3	-.089	1	-.305	1
467	6	max	155.362	4	162.218	3	23.961	1	0	2	0	15	.34	3
468		min	7.376	10	-116.136	1	-.124	3	0	3	-.083	1	-.245	1
469	7	max	147.996	4	263.392	3	56.539	1	0	2	.004	5	.181	3
470		min	7.376	10	-188.72	1	1.455	12	0	3	-.053	1	-.131	1
471	8	max	140.63	4	364.567	3	89.117	1	0	2	.01	4	.038	1
472		min	7.376	10	-261.304	1	2.881	12	0	3	0	3	-.055	3
473	9	max	133.264	4	465.742	3	121.695	1	0	2	.081	1	.261	1
474		min	7.376	10	-333.889	1	4.308	12	0	3	.002	12	-.366	3
475	10	max	125.897	4	566.917	3	154.273	1	0	2	.184	1	.539	1
476		min	7.376	10	-406.473	1	5.735	12	0	3	.006	12	-.754	3
477	11	max	93.159	4	333.888	1	-.552	15	0	3	.078	1	.261	1
478		min	4.093	12	-465.742	3	-121.087	1	0	2	-.014	5	-.366	3
479	12	max	85.793	4	261.304	1	1.131	5	0	3	.002	2	.038	1
480		min	4.093	12	-364.567	3	-88.509	1	0	2	-.014	4	-.055	3
481	13	max	78.426	4	188.72	1	3.406	5	0	3	-.005	12	.181	3
482		min	4.093	12	-263.392	3	-55.931	1	0	2	-.055	1	-.131	1
483	14	max	71.06	4	116.136	1	5.681	5	0	3	-.005	12	.34	3
484		min	4.093	12	-162.218	3	-23.352	1	0	2	-.084	1	-.245	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	68.006	1	43.551	1	10.27	4	0	3	-.002	15	.424	3
486		min	4.093	12	-61.043	3	.433	10	0	2	-.09	1	-.305	1
487	16	max	68.006	1	40.132	3	41.804	1	0	3	.004	5	.432	3
488		min	4.093	12	-29.033	1	3.044	12	0	2	-.07	1	-.31	1
489	17	max	68.006	1	141.307	3	74.382	1	0	3	.012	5	.364	3
490		min	4.093	12	-101.617	1	4.471	12	0	2	-.027	1	-.261	1
491	18	max	68.006	1	242.482	3	106.96	1	0	3	.041	1	.22	3
492		min	4.093	12	-174.201	1	5.897	12	0	2	.003	10	-.158	1
493	19	max	68.006	1	343.657	3	139.538	1	0	3	.134	1	0	1
494		min	4.093	12	-246.786	1	7.324	12	0	2	.009	12	0	3
495	M16	1	max	48.381	5	361.091	2	.033	15	0	.131	1	0	2
496		min	-68.682	1	-167.386	3	-138.858	1	0	2	-.027	5	0	3
497	2	max	41.015	5	254.905	2	1.948	5	0	3	.039	1	.107	3
498		min	-68.682	1	-118.326	3	-106.28	1	0	2	-.026	5	-.231	2
499	3	max	33.648	5	148.719	2	4.223	5	0	3	0	12	.177	3
500		min	-68.682	1	-69.267	3	-73.702	1	0	2	-.03	4	-.382	2
501	4	max	26.282	5	42.533	2	6.498	5	0	3	-.003	12	.211	3
502		min	-68.682	1	-20.207	3	-41.124	1	0	2	-.071	1	-.454	2
503	5	max	18.916	5	28.852	3	8.773	5	0	3	-.005	12	.208	3
504		min	-68.682	1	-63.652	2	-8.546	1	0	2	-.09	1	-.446	2
505	6	max	11.55	5	77.912	3	24.032	1	0	3	-.004	15	.168	3
506		min	-68.682	1	-169.838	2	.347	12	0	2	-.084	1	-.359	2
507	7	max	4.184	5	126.972	3	56.61	1	0	3	.003	5	.091	3
508		min	-68.682	1	-276.024	2	1.774	12	0	2	-.054	1	-.191	2
509	8	max	-1.086	12	176.031	3	89.188	1	0	3	.014	4	.055	2
510		min	-68.682	1	-382.209	2	3.201	12	0	2	-.004	3	-.023	3
511	9	max	-1.086	12	225.091	3	121.766	1	0	3	.08	1	.382	2
512		min	-68.682	1	-488.395	2	4.627	12	0	2	0	3	-.173	3
513	10	max	26.811	5	-11.759	15	154.344	1	0	14	.184	1	.788	2
514		min	-69.599	1	-594.581	2	-9.478	3	0	2	.007	12	-.36	3
515	11	max	19.445	5	488.395	2	-.586	15	0	2	.08	1	.382	2
516		min	-69.599	1	-225.091	3	-121.366	1	0	3	-.012	5	-.173	3
517	12	max	12.078	5	382.209	2	1.078	5	0	2	.002	2	.055	2
518		min	-69.599	1	-176.031	3	-88.788	1	0	3	-.012	4	-.023	3
519	13	max	4.712	5	276.024	2	3.353	5	0	2	-.002	12	.091	3
520		min	-69.599	1	-126.972	3	-56.21	1	0	3	-.054	1	-.191	2
521	14	max	-1.659	15	169.838	2	5.628	5	0	2	-.003	12	.168	3
522		min	-69.599	1	-77.912	3	-23.632	1	0	3	-.084	1	-.359	2
523	15	max	-4.372	12	63.652	2	10.192	4	0	2	-.001	15	.208	3
524		min	-69.599	1	-28.852	3	.444	10	0	3	-.089	1	-.446	2
525	16	max	-4.372	12	20.207	3	41.524	1	0	2	.005	5	.211	3
526		min	-69.599	1	-42.533	2	2.202	12	0	3	-.07	1	-.454	2
527	17	max	-4.372	12	69.267	3	74.102	1	0	2	.014	5	.177	3
528		min	-69.599	1	-148.719	2	3.629	12	0	3	-.027	1	-.382	2
529	18	max	-4.372	12	118.326	3	106.68	1	0	2	.041	1	.107	3
530		min	-69.599	1	-254.905	2	5.056	12	0	3	.003	12	-.231	2
531	19	max	-4.372	12	167.386	3	139.258	1	0	2	.133	1	0	2
532		min	-69.599	1	-361.091	2	6.483	12	0	3	.007	12	0	3
533	M15	1	max	0	2	1.847	1	.05	3	0	1	0	1	1
534		min	-65.259	3	0	2	-.04	1	0	3	0	3	0	1
535	2	max	0	2	1.642	1	.05	3	0	1	0	1	0	2
536		min	-65.334	3	0	2	-.04	1	0	3	0	3	0	1
537	3	max	0	2	1.437	1	.05	3	0	1	0	1	0	2
538		min	-65.41	3	0	2	-.04	1	0	3	0	3	-.001	1
539	4	max	0	2	1.231	1	.05	3	0	1	0	1	0	2
540		min	-65.485	3	0	2	-.04	1	0	3	0	3	-.002	1
541	5	max	0	2	1.026	1	.05	3	0	1	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-65.561	3	0	2	-.04	1	0	3	0	3	-.002	1
543		6	max	0	2	.821	1	.05	3	0	1	0	1	0	2
544			min	-65.637	3	0	2	-.04	1	0	3	0	3	-.003	1
545		7	max	0	2	.616	1	.05	3	0	1	0	3	0	2
546			min	-65.712	3	0	2	-.04	1	0	3	0	1	-.003	1
547		8	max	0	2	.41	1	.05	3	0	1	0	3	0	2
548			min	-65.788	3	0	2	-.04	1	0	3	0	1	-.003	1
549		9	max	0	2	.205	1	.05	3	0	1	0	3	0	2
550			min	-65.863	3	0	2	-.04	1	0	3	0	1	-.003	1
551		10	max	0	2	0	1	.05	3	0	1	0	3	0	2
552			min	-65.939	3	0	1	-.04	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.05	3	0	1	0	3	0	2
554			min	-66.014	3	-.205	1	-.04	1	0	3	0	1	-.003	1
555		12	max	0	2	0	2	.05	3	0	1	0	3	0	2
556			min	-66.09	3	-.41	1	-.04	1	0	3	0	1	-.003	1
557		13	max	0	2	0	2	.05	3	0	1	0	3	0	2
558			min	-66.165	3	-.616	1	-.04	1	0	3	0	1	-.003	1
559		14	max	0	2	0	2	.05	3	0	1	0	3	0	2
560			min	-66.241	3	-.821	1	-.04	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.05	3	0	1	0	3	0	2
562			min	-66.316	3	-1.026	1	-.04	1	0	3	0	1	-.002	1
563		16	max	0	2	0	2	.05	3	0	1	0	3	0	2
564			min	-66.392	3	-1.231	1	-.04	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.05	3	0	1	0	3	0	2
566			min	-66.467	3	-1.437	1	-.04	1	0	3	0	1	-.001	1
567		18	max	0	2	0	2	.05	3	0	1	0	3	0	2
568			min	-66.543	3	-1.642	1	-.04	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.05	3	0	1	0	3	0	1
570			min	-66.618	3	-1.847	1	-.04	1	0	3	0	1	0	1
571	M16A	1	max	-.866	10	3.146	4	.32	4	0	3	0	3	0	1
572			min	-234.383	4	.924	12	-.02	3	0	2	0	4	0	1
573		2	max	-.782	10	2.796	4	.288	4	0	3	0	3	0	12
574			min	-234.429	4	.821	12	-.02	3	0	2	0	4	-.001	4
575		3	max	-.698	10	2.447	4	.255	4	0	3	0	3	0	12
576			min	-234.475	4	.718	12	-.02	3	0	2	0	4	-.002	4
577		4	max	-.614	10	2.097	4	.223	4	0	3	0	3	0	12
578			min	-234.521	4	.616	12	-.02	3	0	2	0	4	-.003	4
579		5	max	-.531	10	1.748	4	.191	4	0	3	0	3	-.001	12
580			min	-234.568	4	.513	12	-.02	3	0	2	0	1	-.004	4
581		6	max	-.447	10	1.398	4	.159	4	0	3	0	5	-.001	12
582			min	-234.614	4	.41	12	-.02	3	0	2	0	1	-.005	4
583		7	max	-.363	10	1.049	4	.126	4	0	3	0	5	-.002	12
584			min	-234.66	4	.308	12	-.02	3	0	2	0	1	-.005	4
585		8	max	-.279	10	.699	4	.094	4	0	3	0	5	-.002	12
586			min	-234.706	4	.205	12	-.02	3	0	2	0	1	-.006	4
587		9	max	-.195	10	.35	4	.062	4	0	3	0	5	-.002	12
588			min	-234.753	4	.103	12	-.02	3	0	2	0	1	-.006	4
589		10	max	-.111	10	0	1	.029	4	0	3	0	5	-.002	12
590			min	-234.799	4	0	1	-.02	3	0	2	0	1	-.006	4
591		11	max	-.027	10	-.103	12	.023	1	0	3	0	5	-.002	12
592			min	-234.845	4	-.35	4	-.02	3	0	2	0	1	-.006	4
593		12	max	.057	10	-.205	12	.023	1	0	3	0	5	-.002	12
594			min	-234.891	4	-.699	4	-.039	5	0	2	0	1	-.006	4
595		13	max	.141	10	-.308	12	.023	1	0	3	0	5	-.002	12
596			min	-234.938	4	-1.049	4	-.071	5	0	2	0	3	-.005	4
597		14	max	.225	10	-.41	12	.023	1	0	3	0	4	-.001	12
598			min	-234.984	4	-1.398	4	-.103	5	0	2	0	3	-.005	4



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 35° 115mph 30psf 6.75ft 7-10Pa Page 32



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.01	5	1.582e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.037e-3	5	NC	1	9633.166	14
45		4	max	0	3	0	2	.015	5	1.815e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-1.053e-3	5	NC	1	6314.433	14
47		5	max	0	3	0	2	.02	5	2.048e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	-1.068e-3	5	NC	1	4667.904	14
49		6	max	0	3	0	2	.025	5	2.281e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-1.084e-3	5	NC	1	3688.928	14
51		7	max	0	3	0	2	.03	4	2.514e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.099e-3	5	NC	1	3042.793	14
53		8	max	0	3	0	2	.035	4	2.747e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.115e-3	5	NC	1	2586.165	14
55		9	max	.001	3	.001	2	.04	4	2.98e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	1	-1.13e-3	5	NC	1	2247.454	14
57		10	max	.001	3	.002	2	.045	4	3.213e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-1.146e-3	5	NC	1	1986.942	14
59		11	max	.001	3	.002	2	.05	4	3.446e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-1.161e-3	5	NC	1	1780.834	14
61		12	max	.002	3	.003	2	.055	4	3.679e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-1.177e-3	5	NC	1	1614.016	14
63		13	max	.002	3	.004	2	.06	4	3.912e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	12	-1.192e-3	5	NC	1	1476.427	14
65		14	max	.002	3	.004	2	.064	4	4.145e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	12	-1.208e-3	5	NC	1	1361.115	14
67		15	max	.002	3	.005	2	.069	4	4.379e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	12	-1.223e-3	5	8579.816	2	1263.12	14
69		16	max	.002	3	.006	2	.074	4	4.612e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	12	-1.239e-3	5	7277.571	2	1178.812	14
71		17	max	.002	3	.007	2	.078	4	4.845e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	12	-1.254e-3	5	6269.042	2	1105.469	14
73		18	max	.002	3	.008	2	.083	4	5.078e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	12	-1.27e-3	5	5478.987	2	1041.01	14
75		19	max	.002	3	.009	2	.087	4	5.311e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	12	-1.285e-3	5	4854.497	2	983.817	14
77	M4	1	max	.002	1	.012	2	0	12	7.036e-3	5	NC	1	NC	3
78			min	0	15	-.011	3	-.092	4	-8.637e-4	1	NC	1	210.456	4
79		2	max	.002	1	.012	2	0	12	7.036e-3	5	NC	1	NC	3
80			min	0	15	-.01	3	-.084	4	-8.637e-4	1	NC	1	229.422	4
81		3	max	.002	1	.011	2	0	12	7.036e-3	5	NC	1	NC	2
82			min	0	15	-.01	3	-.077	4	-8.637e-4	1	NC	1	251.998	4
83		4	max	.001	1	.01	2	0	12	7.036e-3	5	NC	1	NC	2
84			min	0	15	-.009	3	-.069	4	-8.637e-4	1	NC	1	279.132	4
85		5	max	.001	1	.01	2	0	12	7.036e-3	5	NC	1	NC	2
86			min	0	15	-.009	3	-.062	4	-8.637e-4	1	NC	1	312.12	4
87		6	max	.001	1	.009	2	0	12	7.036e-3	5	NC	1	NC	2
88			min	0	15	-.008	3	-.055	4	-8.637e-4	1	NC	1	352.761	4
89		7	max	.001	1	.008	2	0	12	7.036e-3	5	NC	1	NC	2
90			min	0	15	-.007	3	-.048	4	-8.637e-4	1	NC	1	403.621	4
91		8	max	.001	1	.008	2	0	12	7.036e-3	5	NC	1	NC	2
92			min	0	15	-.007	3	-.041	4	-8.637e-4	1	NC	1	468.455	4
93		9	max	0	1	.007	2	0	12	7.036e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.035	4	-8.637e-4	1	NC	1	552.941	4
95		10	max	0	1	.006	2	0	12	7.036e-3	5	NC	1	NC	1
96			min	0	15	-.005	3	-.029	4	-8.637e-4	1	NC	1	666.007	4
97		11	max	0	1	.006	2	0	12	7.036e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.024	4	-8.637e-4	1	NC	1	822.359	4
99		12	max	0	1	.005	2	0	12	7.036e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.018	4	-8.637e-4	1	NC	1	1047.643	4
101		max	0	1	.004	2	0	12	7.036e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.014	4	-8.637e-4	1	NC	1	1389.97	4
103		max	0	1	.003	2	0	12	7.036e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.01	4	-8.637e-4	1	NC	1	1948.575	4
105		max	0	1	.003	2	0	12	7.036e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.007	4	-8.637e-4	1	NC	1	2956.598	4
107		max	0	1	.002	2	0	12	7.036e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.004	4	-8.637e-4	1	NC	1	5076.805	4
109		max	0	1	.001	2	0	12	7.036e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.002	4	-8.637e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	12	7.036e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-8.637e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	7.036e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-8.637e-4	1	NC	1	NC	1
115	M6	max	.008	1	.038	2	.005	1	1.865e-3	4	NC	3	NC	2
116		min	-.012	3	-.035	3	-.016	5	2.828e-7	10	1122.043	2	8815.358	1
117		max	.008	1	.035	2	.004	1	1.884e-3	4	NC	3	NC	2
118		min	-.012	3	-.034	3	-.016	5	-3.041e-7	10	1201.087	2	9525.123	1
119		max	.008	1	.033	2	.004	1	1.903e-3	4	NC	3	NC	1
120		min	-.011	3	-.032	3	-.016	5	-1.937e-6	2	1291.711	2	NC	1
121		max	.007	1	.03	2	.004	1	1.922e-3	4	NC	3	NC	1
122		min	-.01	3	-.03	3	-.016	5	-4.254e-6	2	1396.222	2	NC	1
123		max	.007	1	.028	2	.003	1	1.941e-3	4	NC	3	NC	1
124		min	-.01	3	-.028	3	-.015	5	-6.571e-6	2	1517.59	2	NC	1
125		max	.006	1	.026	2	.003	1	1.96e-3	4	NC	3	NC	1
126		min	-.009	3	-.026	3	-.015	5	-8.888e-6	2	1659.702	2	NC	1
127		max	.006	1	.023	2	.003	1	1.979e-3	4	NC	3	NC	1
128		min	-.008	3	-.024	3	-.014	5	-1.121e-5	2	1827.74	2	NC	1
129		max	.005	1	.021	2	.002	1	1.998e-3	4	NC	3	NC	1
130		min	-.008	3	-.022	3	-.014	5	-1.352e-5	2	2028.771	2	NC	1
131		max	.005	1	.019	2	.002	1	2.017e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.013	5	-1.584e-5	2	2272.684	2	NC	1
133		max	.004	1	.016	2	.002	1	2.036e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.012	5	-1.816e-5	2	2573.753	2	9931.28	4
135		max	.004	1	.014	2	.001	1	2.055e-3	4	NC	3	NC	1
136		min	-.006	3	-.016	3	-.011	5	-2.047e-5	2	2953.375	2	9983.507	4
137		max	.003	1	.012	2	.001	1	2.074e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.01	5	-2.279e-5	2	3445.155	2	NC	1
139		max	.003	1	.01	2	0	1	2.093e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.009	5	-2.511e-5	2	4105.064	2	NC	1
141		max	.002	1	.008	2	0	1	2.112e-3	4	NC	3	NC	1
142		min	-.003	3	-.01	3	-.008	5	-2.742e-5	2	5033.8	2	NC	1
143		max	.002	1	.007	2	0	1	2.131e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.006	5	-3.077e-5	11	6432.682	2	NC	1
145		max	.001	1	.005	2	0	1	2.15e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.005	5	-3.698e-5	1	8771.331	2	NC	1
147		max	0	1	.003	2	0	1	2.169e-3	5	NC	1	NC	1
148		min	-.001	3	-.004	3	-.003	5	-4.618e-5	1	NC	1	NC	1
149		max	0	1	.002	2	0	1	2.189e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-5.538e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	2.21e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-6.458e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	3.054e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.057e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.005	5	2.694e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-1.058e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.011	5	2.334e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.061e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.016	5	1.974e-5	1	NC	1	NC	1
160			min	-.001	2	-.007	3	0	1	-1.064e-3	4	9878.85	2	NC	1
161		5	max	.002	3	.006	2	.021	5	1.614e-5	1	NC	1	NC	1
162			min	-.002	2	-.009	3	0	1	-1.066e-3	4	7453.79	2	NC	1
163		6	max	.002	3	.008	2	.027	5	2.566e-5	3	NC	1	NC	1
164			min	-.002	2	-.011	3	0	1	-1.069e-3	4	5970.08	2	NC	1
165		7	max	.003	3	.009	2	.032	5	4.545e-5	3	NC	3	NC	1
166			min	-.003	2	-.012	3	0	1	-1.072e-3	4	4959.261	2	NC	1
167		8	max	.003	3	.011	2	.037	5	6.524e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	-.001	1	-1.074e-3	4	4221.03	2	NC	1
169		9	max	.003	3	.013	2	.042	5	8.503e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	-.001	1	-1.077e-3	4	3655.486	2	NC	1
171		10	max	.004	3	.014	2	.047	5	1.048e-4	3	NC	3	NC	1
172			min	-.004	2	-.018	3	-.001	1	-1.08e-3	4	3207.215	2	NC	1
173		11	max	.004	3	.016	2	.052	5	1.246e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.083e-3	4	2842.948	2	NC	1
175		12	max	.005	3	.018	2	.056	4	1.444e-4	3	NC	3	NC	1
176			min	-.005	2	-.021	3	-.002	1	-1.085e-3	4	2541.42	2	NC	1
177		13	max	.005	3	.02	2	.061	4	1.642e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.088e-3	4	2288.333	2	NC	1
179		14	max	.006	3	.022	2	.065	4	1.84e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-1.091e-3	4	2073.651	2	NC	1
181		15	max	.006	3	.024	2	.07	4	2.038e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.002	1	-1.093e-3	4	1890.073	2	NC	1
183		16	max	.006	3	.027	2	.074	4	2.236e-4	3	NC	3	NC	1
184			min	-.007	2	-.026	3	-.002	1	-1.096e-3	4	1732.128	2	NC	1
185		17	max	.007	3	.029	2	.079	4	2.433e-4	3	NC	3	NC	1
186			min	-.008	2	-.027	3	-.002	1	-1.099e-3	4	1595.611	2	NC	1
187		18	max	.007	3	.031	2	.083	4	2.631e-4	3	NC	3	NC	1
188			min	-.008	2	-.028	3	-.002	1	-1.101e-3	4	1477.229	2	NC	1
189		19	max	.008	3	.034	2	.087	4	2.829e-4	3	NC	3	NC	1
190			min	-.009	2	-.029	3	-.002	1	-1.104e-3	4	1374.36	2	NC	1
191	M8	1	max	.005	1	.044	2	.002	1	6.859e-3	4	NC	1	NC	2
192			min	0	15	-.035	3	-.092	4	-2.266e-4	3	NC	1	210.35	4
193		2	max	.004	1	.041	2	.002	1	6.859e-3	4	NC	1	NC	2
194			min	0	15	-.033	3	-.084	4	-2.266e-4	3	NC	1	229.306	4
195		3	max	.004	1	.039	2	.002	1	6.859e-3	4	NC	1	NC	2
196			min	0	15	-.031	3	-.077	4	-2.266e-4	3	NC	1	251.869	4
197		4	max	.004	1	.037	2	.002	1	6.859e-3	4	NC	1	NC	1
198			min	0	15	-.029	3	-.069	4	-2.266e-4	3	NC	1	278.989	4
199		5	max	.004	1	.034	2	.002	1	6.859e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.062	4	-2.266e-4	3	NC	1	311.958	4
201		6	max	.003	1	.032	2	.001	1	6.859e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.055	4	-2.266e-4	3	NC	1	352.577	4
203		7	max	.003	1	.029	2	.001	1	6.859e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.048	4	-2.266e-4	3	NC	1	403.41	4
205		8	max	.003	1	.027	2	.001	1	6.859e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.041	4	-2.266e-4	3	NC	1	468.207	4
207		9	max	.003	1	.024	2	0	1	6.859e-3	4	NC	1	NC	1
208			min	0	15	-.019	3	-.035	4	-2.266e-4	3	NC	1	552.647	4
209		10	max	.002	1	.022	2	0	1	6.859e-3	4	NC	1	NC	1
210			min	0	15	-.017	3	-.029	4	-2.266e-4	3	NC	1	665.652	4
211		11	max	.002	1	.019	2	0	1	6.859e-3	4	NC	1	NC	1
212			min	0	15	-.016	3	-.024	4	-2.266e-4	3	NC	1	821.917	4
213		12	max	.002	1	.017	2	0	1	6.859e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	15	-.014	3	-.018	4	-2.266e-4	3	NC	1	1047.078	4
215		13	max	.002	1	.015	2	0	1	6.859e-3	4	NC	1	NC	1
216			min	0	15	-.012	3	-.014	4	-2.266e-4	3	NC	1	1389.216	4
217		14	max	.001	1	.012	2	0	1	6.859e-3	4	NC	1	NC	1
218			min	0	15	-.01	3	-.01	4	-2.266e-4	3	NC	1	1947.513	4
219		15	max	.001	1	.01	2	0	1	6.859e-3	4	NC	1	NC	1
220			min	0	15	-.008	3	-.007	4	-2.266e-4	3	NC	1	2954.979	4
221		16	max	0	1	.007	2	0	1	6.859e-3	4	NC	1	NC	1
222			min	0	15	-.006	3	-.004	4	-2.266e-4	3	NC	1	5074.011	4
223		17	max	0	1	.005	2	0	1	6.859e-3	4	NC	1	NC	1
224			min	0	15	-.004	3	-.002	4	-2.266e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	6.859e-3	4	NC	1	NC	1
226			min	0	15	-.002	3	0	4	-2.266e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	6.859e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.266e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.011	2	0	3	1.004e-3	1	NC	3	NC	1
230			min	-.004	3	-.011	3	-.007	4	-3.985e-4	3	4003.347	2	NC	1
231		2	max	.003	1	.01	2	0	3	9.532e-4	1	NC	3	NC	1
232			min	-.003	3	-.011	3	-.008	4	-3.849e-4	3	4380.316	2	NC	1
233		3	max	.002	1	.009	2	0	3	9.019e-4	1	NC	3	NC	1
234			min	-.003	3	-.01	3	-.008	4	-3.712e-4	3	4830.71	2	NC	1
235		4	max	.002	1	.008	2	0	3	8.506e-4	1	NC	1	NC	1
236			min	-.003	3	-.01	3	-.008	4	-3.575e-4	3	5372.559	2	NC	1
237		5	max	.002	1	.007	2	0	3	8.283e-4	4	NC	1	NC	1
238			min	-.003	3	-.009	3	-.008	4	-3.438e-4	3	6029.98	2	NC	1
239		6	max	.002	1	.006	2	0	3	8.954e-4	4	NC	1	NC	1
240			min	-.003	3	-.009	3	-.008	4	-3.301e-4	3	6835.761	2	NC	1
241		7	max	.002	1	.005	2	0	3	9.625e-4	4	NC	1	NC	1
242			min	-.002	3	-.008	3	-.008	4	-3.164e-4	3	7835.318	2	NC	1
243		8	max	.002	1	.005	2	0	3	1.03e-3	4	NC	1	NC	1
244			min	-.002	3	-.008	3	-.008	4	-3.028e-4	3	9092.963	2	NC	1
245		9	max	.002	1	.004	2	0	3	1.097e-3	4	NC	1	NC	1
246			min	-.002	3	-.007	3	-.008	4	-2.891e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	1.164e-3	4	NC	1	NC	1
248			min	-.002	3	-.007	3	-.008	4	-2.754e-4	3	NC	1	9787.098	4
249		11	max	.001	1	.003	2	0	3	1.231e-3	4	NC	1	NC	1
250			min	-.002	3	-.006	3	-.008	4	-2.617e-4	3	NC	1	9771.077	4
251		12	max	.001	1	.002	2	0	3	1.298e-3	4	NC	1	NC	1
252			min	-.001	3	-.006	3	-.007	4	-2.48e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	1.365e-3	4	NC	1	NC	1
254			min	-.001	3	-.005	3	-.006	4	-2.343e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.432e-3	4	NC	1	NC	1
256			min	0	3	-.004	3	-.006	4	-2.207e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.499e-3	4	NC	1	NC	1
258			min	0	3	-.003	3	-.005	4	-2.07e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.567e-3	4	NC	1	NC	1
260			min	0	3	-.003	3	-.004	4	-1.933e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.634e-3	4	NC	1	NC	1
262			min	0	3	-.002	3	-.003	4	-1.796e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.701e-3	4	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-1.659e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.768e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.522e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.283e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-8.466e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.004	4	5.203e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-9.404e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.009	4	3.122e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.034e-3	4	NC	1	NC	1
273		4	max	0	3	0	2	.013	4	1.042e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.128e-3	4	NC	1	NC	1
275		5	max	0	3	0	2	.017	4	-7.224e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-1.222e-3	4	NC	1	NC	1
277		6	max	0	3	0	2	.021	5	-1.991e-5	12	NC	1	NC	1
278			min	0	2	-.005	3	-.001	3	-1.316e-3	4	NC	1	NC	1
279		7	max	0	3	0	2	.026	5	-2.905e-5	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-1.409e-3	4	NC	1	NC	1
281		8	max	0	3	0	2	.03	5	-3.276e-5	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	1	-1.503e-3	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.034	5	-3.647e-5	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	1	-1.597e-3	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.039	5	-4.018e-5	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.004	1	-1.691e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.043	5	-4.389e-5	10	NC	1	NC	2
288			min	-.001	2	-.008	3	-.005	1	-1.784e-3	4	NC	1	9652.606	1
289		12	max	.002	3	.003	2	.047	5	-4.761e-5	10	NC	1	NC	2
290			min	-.001	2	-.008	3	-.006	1	-1.878e-3	4	NC	1	8017.068	1
291		13	max	.002	3	.004	2	.051	5	-5.132e-5	10	NC	1	NC	2
292			min	-.002	2	-.008	3	-.007	1	-1.972e-3	4	NC	1	6812.205	1
293		14	max	.002	3	.004	2	.055	5	-5.503e-5	10	NC	1	NC	2
294			min	-.002	2	-.009	3	-.008	1	-2.066e-3	4	NC	1	5898.828	1
295		15	max	.002	3	.005	2	.06	5	-5.874e-5	10	NC	1	NC	2
296			min	-.002	2	-.009	3	-.009	1	-2.159e-3	4	8591.743	2	5190.347	1
297		16	max	.002	3	.006	2	.064	5	-6.245e-5	10	NC	1	NC	2
298			min	-.002	2	-.009	3	-.01	1	-2.253e-3	4	7286.83	2	4630.538	1
299		17	max	.002	3	.007	2	.068	5	-6.616e-5	10	NC	1	NC	2
300			min	-.002	2	-.009	3	-.011	1	-2.347e-3	4	6276.417	2	4181.583	1
301		18	max	.002	3	.008	2	.072	5	-6.988e-5	10	NC	1	NC	2
302			min	-.002	2	-.009	3	-.012	1	-2.441e-3	4	5485.008	2	3817.29	1
303		19	max	.002	3	.009	2	.076	5	-7.359e-5	10	NC	3	NC	3
304			min	-.002	2	-.009	3	-.013	1	-2.535e-3	4	4859.528	2	3519.072	1
305	M12	1	max	.002	1	.012	2	.011	1	8.221e-3	4	NC	1	NC	3
306			min	0	15	-.011	3	-.084	5	8.244e-5	10	NC	1	229.699	5
307		2	max	.002	1	.012	2	.01	1	8.221e-3	4	NC	1	NC	3
308			min	0	15	-.01	3	-.077	5	8.244e-5	10	NC	1	250.394	5
309		3	max	.002	1	.011	2	.009	1	8.221e-3	4	NC	1	NC	3
310			min	0	15	-.01	3	-.07	5	8.244e-5	10	NC	1	275.027	5
311		4	max	.001	1	.01	2	.008	1	8.221e-3	4	NC	1	NC	3
312			min	0	15	-.009	3	-.063	5	8.244e-5	10	NC	1	304.632	5
313		5	max	.001	1	.01	2	.007	1	8.221e-3	4	NC	1	NC	3
314			min	0	15	-.009	3	-.057	5	8.244e-5	10	NC	1	340.625	5
315		6	max	.001	1	.009	2	.006	1	8.221e-3	4	NC	1	NC	3
316			min	0	15	-.008	3	-.05	5	8.244e-5	10	NC	1	384.967	5
317		7	max	.001	1	.008	2	.006	1	8.221e-3	4	NC	1	NC	3
318			min	0	15	-.007	3	-.044	5	8.244e-5	10	NC	1	440.458	5
319		8	max	.001	1	.008	2	.005	1	8.221e-3	4	NC	1	NC	2
320			min	0	15	-.007	3	-.038	5	8.244e-5	10	NC	1	511.194	5
321		9	max	0	1	.007	2	.004	1	8.221e-3	4	NC	1	NC	2
322			min	0	15	-.006	3	-.032	5	8.244e-5	10	NC	1	603.37	5
323		10	max	0	1	.006	2	.003	1	8.221e-3	4	NC	1	NC	2
324			min	0	15	-.005	3	-.027	5	8.244e-5	10	NC	1	726.726	5
325		11	max	0	1	.006	2	.003	1	8.221e-3	4	NC	1	NC	2
326			min	0	15	-.005	3	-.022	5	8.244e-5	10	NC	1	897.304	5
327		12	max	0	1	.005	2	.002	1	8.221e-3	4	NC	1	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.017	5	8.244e-5	10	NC	1	1143.083	5
329		max	0	1	.004	2	.002	1	8.221e-3	4	NC	1	NC	1
330		min	0	15	-.004	3	-.013	5	8.244e-5	10	NC	1	1516.546	5
331		max	0	1	.003	2	.001	1	8.221e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.009	5	8.244e-5	10	NC	1	2125.95	5
333		max	0	1	.003	2	0	1	8.221e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.006	5	8.244e-5	10	NC	1	3225.622	5
335		max	0	1	.002	2	0	1	8.221e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.003	5	8.244e-5	10	NC	1	5538.556	5
337		max	0	1	.001	2	0	1	8.221e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.002	5	8.244e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	8.221e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	8.244e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	8.221e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	8.244e-5	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.009	5	1.561e-2	1	NC	1	NC	1
344		min	-.009	2	-.024	2	-.005	1	-2.157e-2	3	NC	1	NC	1
345		max	.01	3	.016	3	.012	5	7.538e-3	2	NC	4	NC	2
346		min	-.009	2	-.014	2	-.01	1	-1.069e-2	3	4918.447	2	8999.253	1
347		max	.01	3	.007	3	.016	5	6.187e-4	5	NC	4	NC	2
348		min	-.009	2	-.005	2	-.013	1	-6.829e-4	1	2524.292	2	5462.176	1
349		max	.01	3	.003	1	.021	5	6.377e-4	5	NC	4	NC	2
350		min	-.009	2	-.002	3	-.015	1	-5.913e-4	1	1765.44	2	3972.562	5
351		max	.009	3	.01	2	.025	5	6.568e-4	5	NC	4	NC	2
352		min	-.009	2	-.008	3	-.015	1	-4.998e-4	1	1398.791	2	2840.641	5
353		max	.009	3	.016	2	.03	5	6.758e-4	5	NC	4	NC	2
354		min	-.009	2	-.014	3	-.014	1	-4.082e-4	1	1190.043	2	2180.736	5
355		max	.009	3	.02	2	.036	5	6.949e-4	5	NC	5	NC	2
356		min	-.009	2	-.018	3	-.013	1	-3.167e-4	1	1062.219	2	1753.883	5
357		max	.009	3	.024	2	.041	5	7.14e-4	5	NC	5	NC	2
358		min	-.009	2	-.021	3	-.01	1	-2.251e-4	1	983.271	2	1458.316	5
359		max	.009	3	.026	2	.047	5	7.33e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.007	1	-1.336e-4	1	938.254	2	1242.218	4
361		max	.009	3	.027	2	.052	5	7.521e-4	5	NC	5	NC	1
362		min	-.009	2	-.024	3	-.004	1	-4.201e-5	1	920.35	2	1061.167	4
363		max	.009	3	.027	2	.058	4	7.852e-4	4	NC	5	NC	1
364		min	-.009	2	-.023	3	-.001	1	1.465e-5	10	927.7	2	925.568	4
365		max	.009	3	.025	2	.064	4	8.264e-4	4	NC	5	NC	2
366		min	-.009	2	-.021	3	0	10	2.403e-5	10	962.746	2	821.69	4
367		max	.009	3	.022	2	.071	4	8.676e-4	4	NC	4	NC	2
368		min	-.009	2	-.018	3	0	10	3.175e-5	12	1033.563	2	740.766	4
369		max	.009	3	.017	2	.077	4	9.088e-4	4	NC	4	NC	2
370		min	-.009	2	-.014	3	0	12	3.551e-5	12	1158.587	2	676.994	4
371		max	.009	3	.01	2	.082	4	9.5e-4	4	NC	4	NC	2
372		min	-.009	2	-.008	3	0	12	3.927e-5	12	1380.917	2	626.422	4
373		max	.009	3	.002	1	.087	4	1.322e-3	4	NC	4	NC	2
374		min	-.009	2	-.002	3	0	12	4.201e-5	12	1820.607	2	586.301	4
375		max	.009	3	.006	3	.092	4	9.58e-3	4	NC	4	NC	2
376		min	-.009	2	-.008	2	0	12	-1.632e-4	1	2628.442	1	554.721	4
377		max	.009	3	.014	3	.096	4	1.13e-2	2	NC	2	NC	2
378		min	-.009	2	-.02	2	0	10	-5.365e-3	3	5081.519	1	530.257	4
379		max	.009	3	.023	3	.099	4	2.287e-2	2	NC	1	NC	1
380		min	-.009	2	-.032	2	-.003	1	-1.086e-2	3	5809.236	2	512.547	4
381	M5	max	.03	3	.088	3	.008	5	1.054e-5	4	NC	1	NC	1
382		min	-.034	2	-.08	2	-.006	1	4.251e-8	10	3512.132	3	NC	1
383		max	.03	3	.053	3	.012	5	3.136e-4	5	NC	5	NC	1
384		min	-.034	2	-.048	2	-.005	1	-5.824e-5	1	1470.691	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.021	3	.016	5	6.117e-4	5	NC	5	NC	1
386		min	-.034	2	-.018	2	-.005	1	-1.16e-4	1	754.373	2	NC	1
387	4	max	.03	3	.009	2	.021	5	6.394e-4	5	NC	5	NC	1
388		min	-.034	2	-.005	3	-.004	1	-1.112e-4	1	527.073	2	NC	1
389	5	max	.03	3	.033	2	.026	5	6.671e-4	5	NC	5	NC	1
390		min	-.034	2	-.028	3	-.004	1	-1.065e-4	1	417.208	2	NC	1
391	6	max	.03	3	.052	2	.032	5	6.948e-4	5	NC	5	NC	1
392		min	-.034	2	-.046	3	-.004	1	-1.017e-4	1	354.629	2	NC	1
393	7	max	.03	3	.068	2	.037	5	7.226e-4	5	NC	15	NC	1
394		min	-.034	2	-.06	3	-.004	1	-9.699e-5	1	316.281	2	NC	1
395	8	max	.03	3	.08	2	.043	5	7.503e-4	5	NC	15	NC	1
396		min	-.034	2	-.07	3	-.003	1	-9.223e-5	1	292.563	2	9948.71	3
397	9	max	.03	3	.088	2	.049	5	7.78e-4	5	NC	15	NC	1
398		min	-.034	2	-.075	3	-.003	1	-8.748e-5	1	278.996	2	NC	1
399	10	max	.03	3	.092	2	.055	5	8.057e-4	5	NC	15	NC	1
400		min	-.034	2	-.077	3	-.003	1	-8.273e-5	1	273.534	2	NC	1
401	11	max	.03	3	.09	2	.061	5	8.335e-4	5	NC	15	NC	1
402		min	-.034	2	-.074	3	-.003	1	-7.798e-5	1	275.617	2	NC	1
403	12	max	.029	3	.084	2	.067	5	8.612e-4	5	NC	15	NC	1
404		min	-.034	2	-.068	3	-.003	1	-7.322e-5	1	285.971	2	NC	1
405	13	max	.029	3	.073	2	.072	5	8.889e-4	5	NC	15	NC	1
406		min	-.034	2	-.058	3	-.003	1	-6.847e-5	1	307.015	2	NC	1
407	14	max	.029	3	.056	2	.078	5	9.166e-4	5	NC	5	NC	1
408		min	-.034	2	-.044	3	-.003	1	-6.372e-5	1	344.282	2	NC	1
409	15	max	.029	3	.034	2	.083	4	9.444e-4	5	NC	5	NC	1
410		min	-.034	2	-.027	3	-.002	1	-5.896e-5	1	410.753	2	NC	1
411	16	max	.029	3	.007	2	.087	4	1.302e-3	5	NC	5	NC	1
412		min	-.034	2	-.006	3	-.002	1	-6.096e-5	1	542.817	2	NC	1
413	17	max	.029	3	.019	3	.092	4	9.556e-3	4	NC	5	NC	1
414		min	-.034	2	-.027	2	-.002	1	-2.237e-4	1	851.264	3	NC	1
415	18	max	.029	3	.046	3	.096	4	4.901e-3	4	NC	5	NC	1
416		min	-.034	2	-.066	2	-.002	1	-1.145e-4	1	1669.537	3	NC	1
417	19	max	.029	3	.074	3	.099	4	2.509e-6	5	NC	3	NC	1
418		min	-.034	2	-.108	2	-.002	1	-4.737e-7	3	1681.937	2	NC	1
419	M9	1	max	.01	.027	3	.007	5	2.158e-2	3	NC	1	NC	1
420		min	-.009	2	-.024	2	-.006	1	-1.561e-2	1	NC	1	NC	1
421	2	max	.01	3	.016	3	.007	5	1.066e-2	3	NC	4	NC	2
422		min	-.009	2	-.014	2	-.001	1	-7.601e-3	2	4920.175	2	9862.87	1
423	3	max	.01	3	.006	3	.007	4	2.777e-4	1	NC	4	NC	2
424		min	-.009	2	-.005	2	0	3	-6.62e-5	3	2525.202	2	6066.281	1
425	4	max	.01	3	.003	2	.009	4	2.002e-4	1	NC	4	NC	2
426		min	-.009	2	-.002	3	-.001	3	-7.046e-5	3	1766.079	2	5092.085	1
427	5	max	.01	3	.01	2	.012	4	1.228e-4	1	NC	4	NC	2
428		min	-.009	2	-.009	3	-.002	3	-7.471e-5	3	1399.281	2	4982.95	1
429	6	max	.009	3	.016	2	.015	4	8.437e-5	4	NC	4	NC	2
430		min	-.009	2	-.014	3	-.003	3	-7.897e-5	3	1190.439	2	4839.668	4
431	7	max	.009	3	.02	2	.019	4	9.87e-5	4	NC	4	NC	2
432		min	-.009	2	-.019	3	-.003	3	-8.323e-5	3	1062.548	2	3474.297	4
433	8	max	.009	3	.024	2	.023	4	1.302e-4	5	NC	5	NC	1
434		min	-.009	2	-.022	3	-.004	3	-1.095e-4	1	983.55	2	2623.417	4
435	9	max	.009	3	.026	2	.028	5	1.616e-4	5	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	3	-1.869e-4	1	938.492	2	2058.034	4
437	10	max	.009	3	.027	2	.034	5	1.931e-4	5	NC	5	NC	1
438		min	-.009	2	-.024	3	-.005	1	-2.643e-4	1	920.55	2	1663.306	4
439	11	max	.009	3	.027	2	.041	5	2.245e-4	5	NC	5	NC	1
440		min	-.009	2	-.023	3	-.008	1	-3.418e-4	1	927.861	2	1376.721	4
441	12	max	.009	3	.025	2	.047	5	2.56e-4	5	NC	5	NC	2



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Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.196	3	.087	1	7.095e-3	2	NC	5	NC	3
500		min	-.099	4	-.394	2	.004	10	-4.953e-3	3	447.606	2	1773.04	1
501	4	max	.002	1	.246	3	.131	1	8.319e-3	2	NC	5	NC	10
502		min	-.099	4	-.497	2	.008	10	-5.773e-3	3	348.27	2	1197.702	1
503	5	max	.002	1	.263	3	.151	1	9.543e-3	2	NC	5	NC	10
504		min	-.099	4	-.529	2	.008	10	-6.592e-3	3	325.89	2	1037.645	1
505	6	max	.002	1	.247	3	.143	1	1.077e-2	2	NC	5	NC	10
506		min	-.099	4	-.491	2	.006	10	-7.411e-3	3	352.926	2	1096.394	1
507	7	max	.002	1	.205	3	.107	1	1.199e-2	2	NC	5	NC	5
508		min	-.099	4	-.397	2	0	10	-8.231e-3	3	444	2	1445.191	1
509	8	max	.002	1	.15	3	.055	1	1.322e-2	2	NC	5	NC	2
510		min	-.099	4	-.273	2	-.009	10	-9.05e-3	3	671.429	2	2736.057	1
511	9	max	.002	1	.098	3	.032	3	1.444e-2	2	NC	4	NC	1
512		min	-.099	4	-.159	2	-.023	2	-9.869e-3	3	1269.759	2	7157.119	3
513	10	max	.002	1	.074	3	.029	3	1.566e-2	2	NC	4	NC	4
514		min	-.099	4	-.108	2	-.034	2	-1.069e-2	3	2137.066	2	6655.428	2
515	11	max	.003	1	.098	3	.028	3	1.444e-2	2	NC	4	NC	1
516		min	-.099	4	-.159	2	-.023	2	-9.868e-3	3	1269.759	2	8442.285	3
517	12	max	.003	1	.15	3	.054	1	1.322e-2	2	NC	5	NC	2
518		min	-.099	4	-.273	2	-.009	10	-9.048e-3	3	671.429	2	2770.073	1
519	13	max	.003	1	.205	3	.106	1	1.199e-2	2	NC	5	NC	5
520		min	-.099	4	-.397	2	0	10	-8.227e-3	3	444	2	1461.282	1
521	14	max	.003	1	.247	3	.141	1	1.077e-2	2	NC	5	NC	5
522		min	-.099	4	-.491	2	.005	15	-7.407e-3	3	352.926	2	1109.187	1
523	15	max	.003	1	.263	3	.149	1	9.545e-3	2	NC	5	NC	5
524		min	-.099	4	-.529	2	.001	15	-6.586e-3	3	325.89	2	1051.363	1
525	16	max	.003	1	.246	3	.128	1	8.321e-3	2	NC	5	NC	3
526		min	-.099	4	-.497	2	-.004	5	-5.766e-3	3	348.27	2	1216.891	1
527	17	max	.003	1	.195	3	.085	1	7.097e-3	2	NC	5	NC	3
528		min	-.099	4	-.394	2	-.008	5	-4.945e-3	3	447.606	2	1810.528	1
529	18	max	.003	1	.118	3	.033	1	5.873e-3	2	NC	5	NC	2
530		min	-.099	4	-.231	2	-.008	5	-4.125e-3	3	814.75	2	4311.243	1
531	19	max	.003	1	.023	3	.009	3	4.649e-3	2	NC	1	NC	1
532		min	-.099	4	-.032	2	-.009	2	-3.304e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.061e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.235e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.011	4	9.259e-4	3	NC	3	NC	1
536		min	0	5	-.011	1	0	3	-7.49e-4	5	7992.014	2	8271.858	4
537	3	max	0	3	0	15	.025	4	1.446e-3	3	NC	5	NC	1
538		min	-.002	5	-.023	1	-.004	3	-1.129e-3	2	4066.863	2	3680.672	4
539	4	max	0	3	0	15	.04	4	1.965e-3	3	NC	5	NC	9
540		min	-.002	5	-.033	1	-.008	3	-1.662e-3	2	2790.106	2	2308.85	4
541	5	max	0	3	0	15	.054	4	2.485e-3	3	NC	5	NC	9
542		min	-.003	5	-.042	1	-.013	3	-2.195e-3	2	2177.148	2	1698.286	4
543	6	max	0	3	0	15	.066	4	3.005e-3	3	NC	5	9977.038	9
544		min	-.004	5	-.05	1	-.019	3	-2.728e-3	2	1832.3	2	1378.514	4
545	7	max	0	3	0	15	.076	4	3.525e-3	3	NC	5	7922.298	9
546		min	-.005	5	-.056	1	-.025	3	-3.261e-3	2	1624.92	2	1201.328	4
547	8	max	0	3	0	15	.082	4	4.044e-3	3	NC	5	6611.605	9
548		min	-.006	5	-.061	1	-.031	3	-3.794e-3	2	1500.46	2	1108.32	4
549	9	max	0	3	0	15	.085	4	4.564e-3	3	NC	5	5746.038	9
550		min	-.007	5	-.064	1	-.037	3	-4.327e-3	2	1433.469	2	1074.833	4
551	10	max	0	3	.001	15	.084	4	5.084e-3	3	NC	5	5173.026	9
552		min	-.007	5	-.065	1	-.041	3	-4.86e-3	2	1412.276	2	1092.811	4
553	11	max	0	3	.001	15	.078	4	5.604e-3	3	NC	5	4811.976	9
554		min	-.008	5	-.064	1	-.044	3	-5.393e-3	2	1433.469	2	1166.03	4
555	12	max	0	3	.002	15	.07	4	6.123e-3	3	NC	5	4623.651	9



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.009	5	-0.061	1	-0.044	3	-5.926e-3	2	1500.46	2	1312.008	4
557	13	max	.001	3	.002	15	.058	4	6.643e-3	3	NC	5	4599.033	9
558		min	-.01	5	-.056	1	-.043	3	-6.459e-3	2	1624.92	2	1343.727	3
559	14	max	.001	3	.003	5	.045	4	7.163e-3	3	NC	5	4761.425	9
560		min	-.011	5	-.05	1	-.039	3	-6.992e-3	2	1832.3	2	1386.44	3
561	15	max	.001	3	.003	5	.03	4	7.683e-3	3	NC	5	5674.545	15
562		min	-.011	5	-.042	1	-.031	3	-7.525e-3	2	2177.148	2	1506.058	3
563	16	max	.001	3	.004	5	.019	1	8.202e-3	3	NC	5	NC	15
564		min	-.012	5	-.033	1	-.02	3	-8.058e-3	2	2790.106	2	1761.255	3
565	17	max	.001	3	.005	5	.008	1	8.722e-3	3	NC	5	NC	4
566		min	-.013	5	-.023	1	-.006	3	-8.591e-3	2	4066.863	2	2335.985	3
567	18	max	.002	3	.006	5	.014	3	9.242e-3	3	NC	3	NC	4
568		min	-.014	5	-.012	9	-.016	2	-9.124e-3	2	7992.014	2	4160.655	3
569	19	max	.002	3	.007	5	.038	3	9.762e-3	3	NC	1	NC	1
570		min	-.015	5	-.003	9	-.038	2	-9.658e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.011	3	2.858e-3	3	NC	1	NC	1
572		min	-.006	4	-.004	4	-.011	2	-2.679e-3	2	NC	1	NC	1
573	2	max	0	10	-.006	12	.004	9	2.746e-3	3	NC	3	NC	1
574		min	-.005	4	-.024	4	-.005	5	-2.563e-3	2	4693.191	4	NC	1
575	3	max	0	10	-.012	12	.01	1	2.634e-3	3	8133.727	12	NC	4
576		min	-.005	4	-.042	4	-.013	5	-2.446e-3	2	2388.205	4	5766.723	1
577	4	max	0	10	-.017	12	.016	1	2.521e-3	3	5580.212	12	NC	10
578		min	-.005	4	-.06	4	-.026	5	-2.33e-3	2	1638.448	4	3738.822	5
579	5	max	0	10	-.021	12	.019	1	2.409e-3	3	4354.296	12	9549.905	10
580		min	-.004	4	-.075	4	-.041	5	-2.213e-3	2	1278.498	4	2325.265	5
581	6	max	0	10	-.025	12	.022	1	2.297e-3	3	3664.6	12	8927.306	10
582		min	-.004	4	-.088	4	-.056	5	-2.097e-3	2	1075.991	4	1677.513	5
583	7	max	0	10	-.029	12	.022	1	2.185e-3	3	3249.839	12	8808.466	10
584		min	-.004	4	-.099	4	-.07	5	-1.981e-3	2	954.21	4	1333.157	5
585	8	max	0	10	-.031	12	.022	1	2.073e-3	3	3000.92	12	9080.49	10
586		min	-.003	4	-.107	4	-.082	5	-1.864e-3	2	881.123	4	1137.167	5
587	9	max	0	10	-.032	12	.021	1	1.961e-3	3	2866.937	12	9737.816	10
588		min	-.003	4	-.111	4	-.09	5	-1.748e-3	2	841.783	4	1026.404	5
589	10	max	0	10	-.033	12	.019	1	1.849e-3	3	2824.552	12	NC	10
590		min	-.003	4	-.112	4	-.095	5	-1.631e-3	2	829.338	4	972.723	5
591	11	max	0	10	-.032	12	.017	1	1.737e-3	3	2866.937	12	NC	10
592		min	-.003	4	-.111	4	-.096	5	-1.515e-3	2	841.783	4	964.447	5
593	12	max	0	10	-.031	12	.014	1	1.625e-3	3	3000.92	12	NC	9
594		min	-.002	4	-.106	4	-.092	5	-1.399e-3	2	881.123	4	1000.138	5
595	13	max	0	10	-.028	12	.011	1	1.512e-3	3	3249.839	12	NC	9
596		min	-.002	4	-.097	4	-.085	5	-1.282e-3	2	954.21	4	1088.033	5
597	14	max	0	10	-.025	12	.008	1	1.4e-3	3	3664.6	12	NC	2
598		min	-.002	4	-.086	4	-.074	5	-1.166e-3	2	1075.991	4	1250.727	5
599	15	max	0	10	-.021	12	.005	1	1.288e-3	3	4354.296	12	NC	1
600		min	-.001	4	-.073	4	-.06	5	-1.049e-3	2	1278.498	4	1540.546	5
601	16	max	0	10	-.016	12	.003	1	1.176e-3	3	5580.212	12	NC	1
602		min	0	4	-.057	4	-.044	5	-9.33e-4	2	1638.448	4	2088.98	5
603	17	max	0	10	-.011	12	.001	9	1.064e-3	3	8133.727	12	NC	1
604		min	0	4	-.039	4	-.028	5	-8.166e-4	2	2388.205	4	3310.481	5
605	18	max	0	10	-.006	12	0	3	1.092e-3	4	NC	3	NC	1
606		min	0	4	-.02	4	-.013	5	-7.002e-4	2	4693.191	4	7355.616	5
607	19	max	0	1	0	1	0	1	1.166e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.838e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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