

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R =	1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s =	0	
S_1 =	0.00	ρ =	1.3	
S_{D1} =	0.00	Ω =	1.25	
T_a =	0.00	C_d =	1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

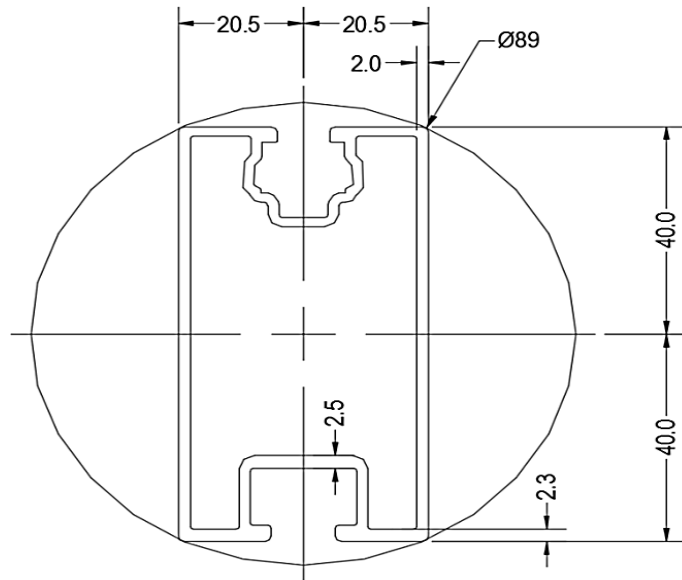
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

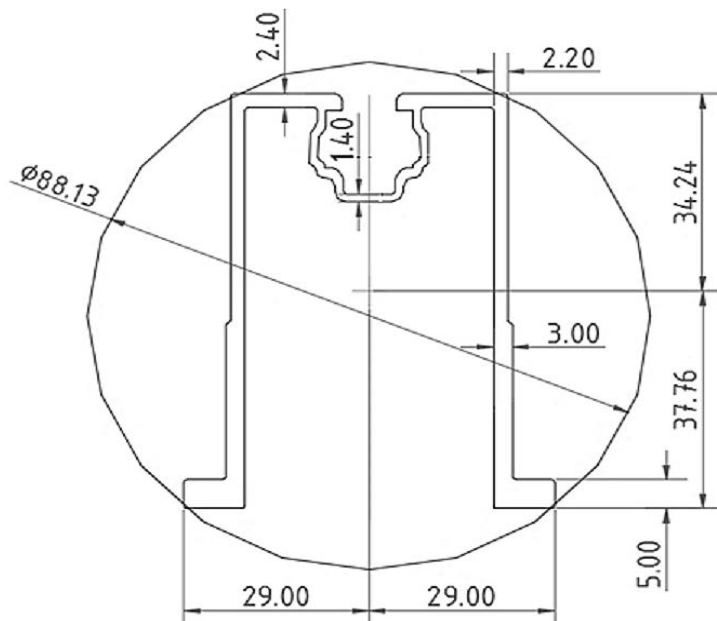
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	102 in
ΦF_{ty} STRONG-AXIS =	28.61 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.234 k-ft
M_z =	0.235 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	97%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

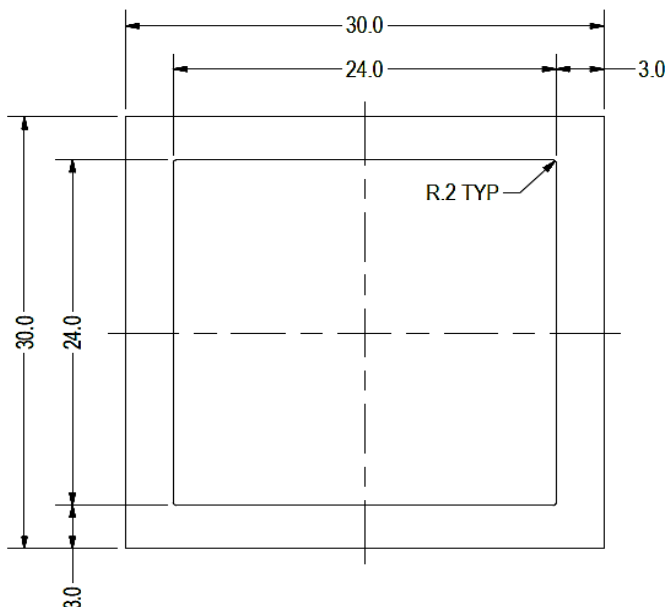
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.98 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.601 k-ft
M_z =	0.000 k-ft
P_n =	0.235 k
$M_{y \text{ allowable}}$ =	1.471 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	43%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

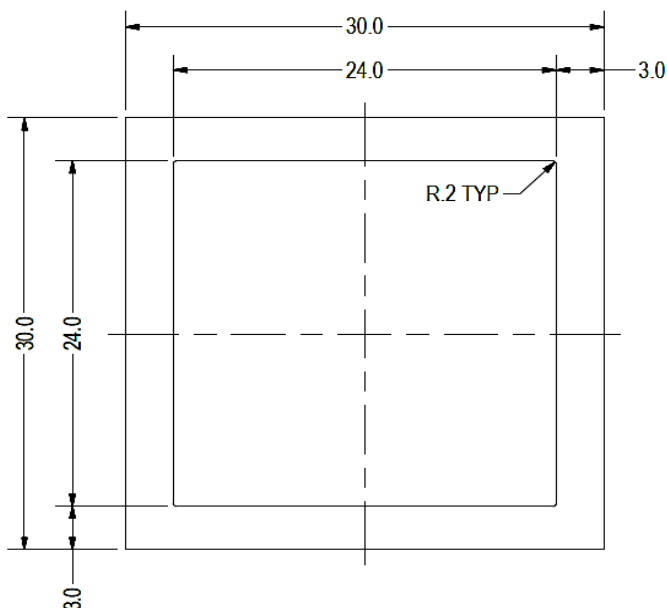
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.603 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	13%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.309 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.311 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	19%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.007 k-ft
P_n =	0.041 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	16%



A cross brace kit is required every 12 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

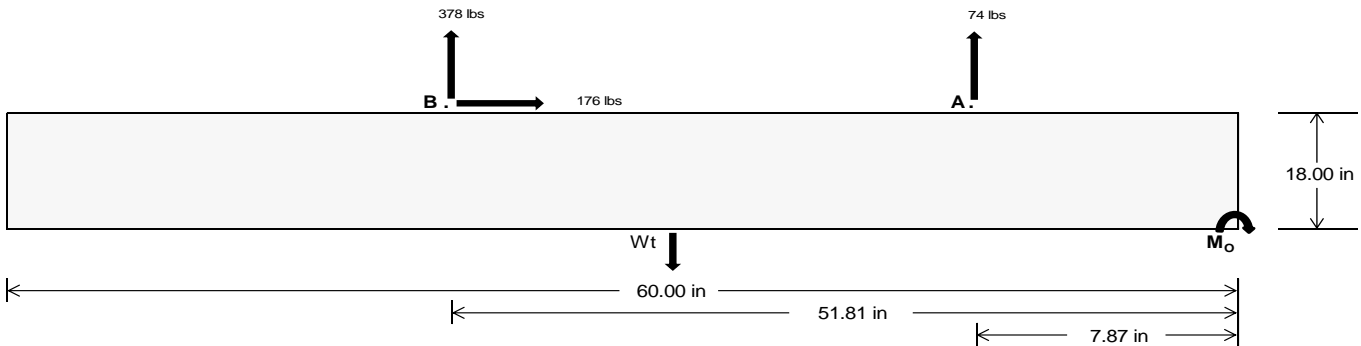
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	314.50	1577.14	k
Compressive Load =	2084.18	1613.10	k
Lateral Load =	5.33	733.67	k
Moment (Weak Axis) =	0.01	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23347.9$ in-lbs
Resisting Force Required = 778.26 lbs
S.F. = 1.67
Weight Required = 1297.10 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 176.28 lbs
Friction = 0.4
Weight Required = 440.69 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 176.28 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	810 lbs	810 lbs	810 lbs	810 lbs	564 lbs	564 lbs	564 lbs	564 lbs	971 lbs	971 lbs	971 lbs	971 lbs	-148 lbs	-148 lbs	-148 lbs	-148 lbs
F_B	595 lbs	595 lbs	595 lbs	595 lbs	497 lbs	497 lbs	497 lbs	497 lbs	773 lbs	773 lbs	773 lbs	773 lbs	-756 lbs	-756 lbs	-756 lbs	-756 lbs
F_V	74 lbs	74 lbs	74 lbs	74 lbs	319 lbs	319 lbs	319 lbs	319 lbs	290 lbs	290 lbs	290 lbs	290 lbs	-353 lbs	-353 lbs	-353 lbs	-353 lbs
P_{total}	3308 lbs	3399 lbs	3490 lbs	3580 lbs	2964 lbs	3055 lbs	3145 lbs	3236 lbs	3647 lbs	3738 lbs	3829 lbs	3919 lbs	237 lbs	292 lbs	346 lbs	400 lbs
M	524 lbs-ft	524 lbs-ft	524 lbs-ft	524 lbs-ft	617 lbs-ft	617 lbs-ft	617 lbs-ft	617 lbs-ft	820 lbs-ft	820 lbs-ft	820 lbs-ft	820 lbs-ft	572 lbs-ft	572 lbs-ft	572 lbs-ft	572 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	0.22 ft	0.22 ft	0.21 ft	0.21 ft	2.41 ft	1.96 ft	1.65 ft	1.43 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	306.3 psf	302.2 psf	298.5 psf	295.2 psf	254.2 psf	252.5 psf	251.0 psf	249.6 psf	304.4 psf	300.5 psf	296.9 psf	293.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	449.9 psf	439.4 psf	429.7 psf	420.9 psf	423.3 psf	414.0 psf	405.4 psf	397.6 psf	529.2 psf	515.1 psf	502.1 psf	490.3 psf	1019.7 psf	197.1 psf	142.2 psf	124.6 psf

Maximum Bearing Pressure = 1020 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

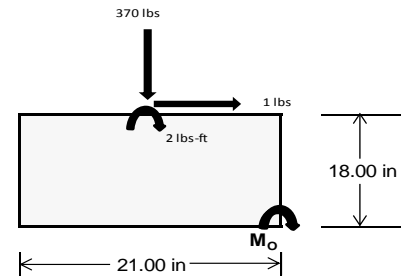
Overturning Check

$M_o = 320.1 \text{ ft-lbs}$
 Resisting Force Required = 365.83 lbs
 S.F. = 1.67
 Weight Required = 609.72 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	94 lbs	261 lbs	89 lbs	375 lbs	1144 lbs	370 lbs	28 lbs	76 lbs	26 lbs
F_v	5 lbs	5 lbs	0 lbs	24 lbs	22 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2450 lbs	2617 lbs	2445 lbs	2618 lbs	3387 lbs	2613 lbs	717 lbs	765 lbs	715 lbs
M	7 lbs-ft	7 lbs-ft	0 lbs-ft	40 lbs-ft	34 lbs-ft	4 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.02 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.74 ft	1.75 ft	1.72 ft	1.73 ft	1.75 ft	1.74 ft	1.74 ft	1.75 ft
f_{min}	277.1 sqft	296.3 sqft	279.3 sqft	283.4 sqft	373.9 sqft	297.2 sqft	81.0 sqft	86.7 sqft	81.7 sqft
f_{max}	283.0 psf	301.9 psf	279.6 psf	315.0 psf	400.3 psf	300.1 psf	82.7 psf	88.3 psf	81.8 psf



Maximum Bearing Pressure = 400 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

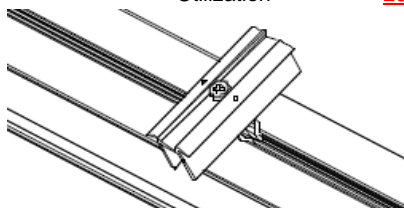
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

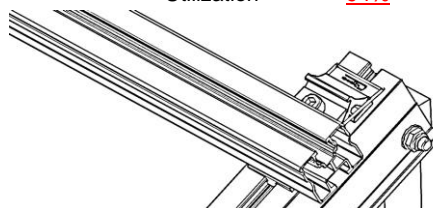
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.355 k
Allowable Uplift =	1.214 k
Utilization =	<u>29%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.052 k
Allowable Uplift =	1.116 k
Utilization =	<u>94%</u>



6.2 Bolted Connections

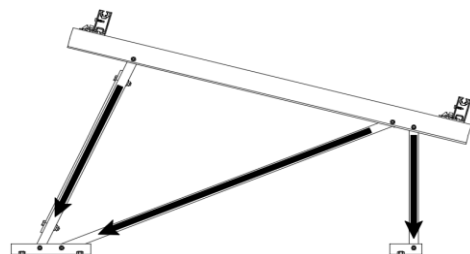
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.603 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>28%</u>

Diagonal Strut

Maximum Axial Load =	0.309 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>5%</u>



Rear Strut

Maximum Axial Load =	1.311 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

Bracing

Maximum Axial Load =	0.041 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.068 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.49 \\ &20.14 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 30.0 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.49 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 30.0 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.471 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	128.755	2	337.396	1	.002	10	0	2	0	1	0	1
2		min	-176.763	3	-368.968	3	-.195	1	0	3	0	1	0	1
3	N7	max	0	15	563	1	-.072	15	0	15	0	1	0	1
4		min	-.197	1	-64.913	3	-1.866	1	-.003	1	0	1	0	1
5	N15	max	-.001	15	1603.214	1	.615	1	.001	1	0	1	0	1
6		min	-2.122	1	-241.924	3	-.242	3	0	3	0	1	0	1
7	N16	max	538.649	2	1240.848	1	-.29	10	0	1	0	1	0	1
8		min	-564.363	3	-1213.183	3	-37.883	1	0	3	0	1	0	1
9	N23	max	0	15	562.776	1	4.103	1	.007	1	0	1	0	1
10		min	-.197	1	-64.465	3	.149	15	0	15	0	1	0	1
11	N24	max	129.256	2	343.246	1	35.225	1	.002	1	0	1	0	1
12		min	-176.809	3	-365.889	3	.051	10	0	3	0	1	0	1
13	Totals:	max	794.611	2	4650.48	1	0	3						
14		min	-918.21	3	-2319.341	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	398.596	1	.642	4	.871	1	0	15	0	3	0	1
2			min	-349.106	3	.152	15	-.032	3	-.001	1	0	1	0	1
3		2	max	398.703	1	.601	4	.871	1	0	15	0	1	0	15
4			min	-349.026	3	.142	15	-.032	3	-.001	1	0	10	0	4
5		3	max	398.809	1	.56	4	.871	1	0	15	0	1	0	15
6			min	-348.946	3	.133	15	-.032	3	-.001	1	0	12	0	4
7		4	max	398.916	1	.519	4	.871	1	0	15	0	1	0	15
8			min	-348.866	3	.123	15	-.032	3	-.001	1	0	3	0	4
9		5	max	399.022	1	.477	4	.871	1	0	15	0	1	0	15
10			min	-348.786	3	.113	15	-.032	3	-.001	1	0	3	0	4
11		6	max	399.129	1	.436	4	.871	1	0	15	0	1	0	15
12			min	-348.706	3	.104	15	-.032	3	-.001	1	0	3	0	4
13		7	max	399.235	1	.395	4	.871	1	0	15	0	1	0	15
14			min	-348.626	3	.094	15	-.032	3	-.001	1	0	3	0	4
15		8	max	399.342	1	.354	4	.871	1	0	15	0	1	0	15
16			min	-348.546	3	.084	15	-.032	3	-.001	1	0	3	0	4
17		9	max	399.448	1	.312	4	.871	1	0	15	0	1	0	15
18			min	-348.466	3	.075	15	-.032	3	-.001	1	0	3	0	4
19		10	max	399.555	1	.271	4	.871	1	0	15	.001	1	0	15
20			min	-348.387	3	.065	15	-.032	3	-.001	1	0	3	0	4
21		11	max	399.662	1	.23	4	.871	1	0	15	.001	1	0	15
22			min	-348.307	3	.055	15	-.032	3	-.001	1	0	3	0	4
23		12	max	399.768	1	.189	4	.871	1	0	15	.001	1	0	15
24			min	-348.227	3	.045	15	-.032	3	-.001	1	0	3	0	4
25		13	max	399.875	1	.147	4	.871	1	0	15	.002	1	0	15
26			min	-348.147	3	.036	15	-.032	3	-.001	1	0	3	0	4
27		14	max	399.981	1	.106	4	.871	1	0	15	.002	1	0	15
28			min	-348.067	3	.026	15	-.032	3	-.001	1	0	3	0	4
29		15	max	400.088	1	.07	2	.871	1	0	15	.002	1	0	15
30			min	-347.987	3	.014	12	-.032	3	-.001	1	0	3	0	4
31		16	max	400.194	1	.038	2	.871	1	0	15	.002	1	0	15
32			min	-347.907	3	-.006	9	-.032	3	-.001	1	0	3	0	4
33		17	max	400.301	1	.009	10	.871	1	0	15	.002	1	0	15
34			min	-347.827	3	-.035	1	-.032	3	-.001	1	0	3	0	4
35		18	max	400.407	1	-.013	15	.871	1	0	15	.002	1	0	15
36			min	-347.747	3	-.067	1	-.032	3	-.001	1	0	3	0	4
37		19	max	400.514	1	-.022	15	.871	1	0	15	.002	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-347.667	3	-.1	4	-.032	3	-.001	1	0	3	0	4
39	M3	1	max	58.955	10	1.795	4	-.028	15	0	15	.003	1	0	4
40			min	-108.672	9	.423	15	-.847	1	0	1	0	15	0	15
41		2	max	58.899	10	1.617	4	-.028	15	0	15	.002	1	0	4
42			min	-108.729	9	.381	15	-.847	1	0	1	0	15	0	15
43		3	max	58.842	10	1.439	4	-.028	15	0	15	.002	1	0	2
44			min	-108.786	9	.339	15	-.847	1	0	1	0	15	0	9
45		4	max	58.786	10	1.262	4	-.028	15	0	15	.002	1	0	15
46			min	-108.842	9	.297	15	-.847	1	0	1	0	15	0	4
47		5	max	58.729	10	1.084	4	-.028	15	0	15	.002	1	0	15
48			min	-108.899	9	.256	15	-.847	1	0	1	0	15	0	4
49		6	max	58.673	10	.906	4	-.028	15	0	15	.002	1	0	15
50			min	-108.955	9	.214	15	-.847	1	0	1	0	15	0	4
51		7	max	58.616	10	.729	4	-.028	15	0	15	.001	1	0	15
52			min	-109.012	9	.172	15	-.847	1	0	1	0	15	0	4
53		8	max	58.559	10	.551	4	-.028	15	0	15	.001	1	0	15
54			min	-109.068	9	.13	15	-.847	1	0	1	0	15	0	4
55		9	max	58.503	10	.373	4	-.028	15	0	15	.001	1	0	15
56			min	-109.125	9	.089	15	-.847	1	0	1	0	15	-.001	4
57		10	max	58.446	10	.196	4	-.028	15	0	15	0	1	0	15
58			min	-109.181	9	.047	15	-.847	1	0	1	0	15	-.001	4
59		11	max	58.39	10	.028	2	-.028	15	0	15	0	1	0	15
60			min	-109.238	9	-.003	9	-.847	1	0	1	0	15	-.001	4
61		12	max	58.333	10	-.037	15	-.028	15	0	15	0	1	0	15
62			min	-109.295	9	-.16	4	-.847	1	0	1	0	15	-.001	4
63		13	max	58.277	10	-.078	15	-.028	15	0	15	0	1	0	15
64			min	-109.351	9	-.337	4	-.847	1	0	1	0	12	-.001	4
65		14	max	58.22	10	-.12	15	-.028	15	0	15	0	1	0	15
66			min	-109.408	9	-.515	4	-.847	1	0	1	0	12	-.001	4
67		15	max	58.164	10	-.162	15	-.028	15	0	15	0	1	0	15
68			min	-109.464	9	-.693	4	-.847	1	0	1	0	3	0	4
69		16	max	58.107	10	-.204	15	-.028	15	0	15	0	15	0	15
70			min	-109.521	9	-.87	4	-.847	1	0	1	0	1	0	4
71		17	max	58.05	10	-.246	15	-.028	15	0	15	0	15	0	15
72			min	-109.577	9	-1.048	4	-.847	1	0	1	0	1	0	4
73		18	max	57.994	10	-.287	15	-.028	15	0	15	0	15	0	15
74			min	-109.634	9	-1.226	4	-.847	1	0	1	0	1	0	4
75		19	max	57.937	10	-.329	15	-.028	15	0	15	0	15	0	1
76			min	-109.69	9	-1.403	4	-.847	1	0	1	0	1	0	1
77	M4	1	max	561.836	1	0	1	-.072	15	0	1	0	3	0	1
78			min	-65.786	3	0	1	-2.058	1	0	1	0	1	0	1
79		2	max	561.9	1	0	1	-.072	15	0	1	0	12	0	1
80			min	-65.738	3	0	1	-2.058	1	0	1	0	1	0	1
81		3	max	561.965	1	0	1	-.072	15	0	1	0	15	0	1
82			min	-65.689	3	0	1	-2.058	1	0	1	0	1	0	1
83		4	max	562.03	1	0	1	-.072	15	0	1	0	15	0	1
84			min	-65.641	3	0	1	-2.058	1	0	1	0	1	0	1
85		5	max	562.095	1	0	1	-.072	15	0	1	0	15	0	1
86			min	-65.592	3	0	1	-2.058	1	0	1	0	1	0	1
87		6	max	562.159	1	0	1	-.072	15	0	1	0	15	0	1
88			min	-65.544	3	0	1	-2.058	1	0	1	0	1	0	1
89		7	max	562.224	1	0	1	-.072	15	0	1	0	15	0	1
90			min	-65.495	3	0	1	-2.058	1	0	1	-.001	1	0	1
91		8	max	562.289	1	0	1	-.072	15	0	1	0	15	0	1
92			min	-65.446	3	0	1	-2.058	1	0	1	-.001	1	0	1
93		9	max	562.353	1	0	1	-.072	15	0	1	0	15	0	1
94			min	-65.398	3	0	1	-2.058	1	0	1	-.002	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	562.418	1	0	1	-.072	15	0	1	0	15	0	1
96		min	-65.349	3	0	1	-2.058	1	0	1	-.002	1	0	1
97	11	max	562.483	1	0	1	-.072	15	0	1	0	15	0	1
98		min	-65.301	3	0	1	-2.058	1	0	1	-.002	1	0	1
99	12	max	562.547	1	0	1	-.072	15	0	1	0	15	0	1
100		min	-65.252	3	0	1	-2.058	1	0	1	-.002	1	0	1
101	13	max	562.612	1	0	1	-.072	15	0	1	0	15	0	1
102		min	-65.204	3	0	1	-2.058	1	0	1	-.002	1	0	1
103	14	max	562.677	1	0	1	-.072	15	0	1	0	15	0	1
104		min	-65.155	3	0	1	-2.058	1	0	1	-.002	1	0	1
105	15	max	562.742	1	0	1	-.072	15	0	1	0	15	0	1
106		min	-65.107	3	0	1	-2.058	1	0	1	-.003	1	0	1
107	16	max	562.806	1	0	1	-.072	15	0	1	0	15	0	1
108		min	-65.058	3	0	1	-2.058	1	0	1	-.003	1	0	1
109	17	max	562.871	1	0	1	-.072	15	0	1	0	15	0	1
110		min	-65.01	3	0	1	-2.058	1	0	1	-.003	1	0	1
111	18	max	562.936	1	0	1	-.072	15	0	1	0	15	0	1
112		min	-64.961	3	0	1	-2.058	1	0	1	-.003	1	0	1
113	19	max	563	1	0	1	-.072	15	0	1	0	15	0	1
114		min	-64.913	3	0	1	-2.058	1	0	1	-.003	1	0	1
115	M6	1	max 1308.611	1	.637	4	.287	1	0	1	0	3	0	1
116		min	-1144.718	3	.151	15	-.107	3	0	15	0	2	0	1
117	2	max	1308.717	1	.595	4	.287	1	0	1	0	3	0	15
118		min	-1144.638	3	.142	15	-.107	3	0	15	0	2	0	4
119	3	max	1308.824	1	.554	4	.287	1	0	1	0	1	0	15
120		min	-1144.558	3	.132	15	-.107	3	0	15	0	15	0	4
121	4	max	1308.93	1	.513	4	.287	1	0	1	0	1	0	15
122		min	-1144.478	3	.122	15	-.107	3	0	15	0	12	0	4
123	5	max	1309.037	1	.471	4	.287	1	0	1	0	1	0	15
124		min	-1144.398	3	.112	15	-.107	3	0	15	0	3	0	4
125	6	max	1309.143	1	.43	4	.287	1	0	1	0	1	0	15
126		min	-1144.318	3	.103	15	-.107	3	0	15	0	3	0	4
127	7	max	1309.25	1	.389	4	.287	1	0	1	0	1	0	15
128		min	-1144.238	3	.093	15	-.107	3	0	15	0	3	0	4
129	8	max	1309.356	1	.348	4	.287	1	0	1	0	1	0	15
130		min	-1144.158	3	.083	15	-.107	3	0	15	0	3	0	4
131	9	max	1309.463	1	.311	2	.287	1	0	1	0	1	0	15
132		min	-1144.078	3	.074	15	-.107	3	0	15	0	3	0	4
133	10	max	1309.569	1	.279	2	.287	1	0	1	0	1	0	15
134		min	-1143.998	3	.064	15	-.107	3	0	15	0	3	0	4
135	11	max	1309.676	1	.247	2	.287	1	0	1	0	1	0	15
136		min	-1143.919	3	.052	12	-.107	3	0	15	0	3	0	4
137	12	max	1309.782	1	.215	2	.287	1	0	1	0	1	0	15
138		min	-1143.839	3	.036	12	-.107	3	0	15	0	3	0	4
139	13	max	1309.889	1	.182	2	.287	1	0	1	0	1	0	15
140		min	-1143.759	3	.02	12	-.107	3	0	15	0	3	0	4
141	14	max	1309.996	1	.15	2	.287	1	0	1	0	1	0	15
142		min	-1143.679	3	.002	3	-.107	3	0	15	0	3	0	4
143	15	max	1310.102	1	.118	2	.287	1	0	1	0	1	0	15
144		min	-1143.599	3	-.023	3	-.107	3	0	15	0	3	0	4
145	16	max	1310.209	1	.086	2	.287	1	0	1	0	1	0	15
146		min	-1143.519	3	-.047	3	-.107	3	0	15	0	3	0	2
147	17	max	1310.315	1	.054	2	.287	1	0	1	0	1	0	15
148		min	-1143.439	3	-.071	3	-.107	3	0	15	0	3	0	2
149	18	max	1310.422	1	.022	2	.287	1	0	1	0	1	0	15
150		min	-1143.359	3	-.095	3	-.107	3	0	15	0	3	0	2
151	19	max	1310.528	1	-.011	2	.287	1	0	1	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1143.279	3	-.119	3	-.107	3	0	15	0	3	0	2
153	M7	1	max	308.829	2	1.793	4	.018	1	0	2	0	2	2
154		min	-256.324	3	.423	15	-.002	3	0	3	0	3	0	12
155		2	max	308.761	2	1.616	4	.018	1	0	2	0	2	2
156		min	-256.375	3	.381	15	-.002	3	0	3	0	3	0	12
157		3	max	308.694	2	1.438	4	.018	1	0	2	0	2	2
158		min	-256.426	3	.339	15	-.002	3	0	3	0	3	0	3
159		4	max	308.626	2	1.26	4	.018	1	0	2	0	2	2
160		min	-256.477	3	.297	15	-.002	3	0	3	0	3	0	3
161		5	max	308.558	2	1.083	4	.018	1	0	2	0	2	15
162		min	-256.527	3	.255	15	-.002	3	0	3	0	3	0	4
163		6	max	308.49	2	.905	4	.018	1	0	2	0	2	15
164		min	-256.578	3	.214	15	-.002	3	0	3	0	3	0	4
165		7	max	308.422	2	.727	4	.018	1	0	2	0	2	15
166		min	-256.629	3	.172	15	-.002	3	0	3	0	3	0	4
167		8	max	308.354	2	.55	4	.018	1	0	2	0	2	15
168		min	-256.68	3	.13	15	-.002	3	0	3	0	3	-.001	4
169		9	max	308.286	2	.372	4	.018	1	0	2	0	2	15
170		min	-256.731	3	.088	15	-.002	3	0	3	0	3	-.001	4
171		10	max	308.219	2	.213	2	.018	1	0	2	0	2	15
172		min	-256.782	3	.043	12	-.002	3	0	3	0	3	-.001	4
173		11	max	308.151	2	.074	2	.018	1	0	2	0	2	15
174		min	-256.833	3	-.043	3	-.002	3	0	3	0	3	-.001	4
175		12	max	308.083	2	-.037	15	.018	1	0	2	0	2	15
176		min	-256.884	3	-.161	4	-.002	3	0	3	0	3	-.001	4
177		13	max	308.015	2	-.079	15	.018	1	0	2	0	2	15
178		min	-256.935	3	-.338	4	-.002	3	0	3	0	3	-.001	4
179		14	max	307.947	2	-.12	15	.018	1	0	2	0	2	15
180		min	-256.986	3	-.516	4	-.002	3	0	3	0	3	-.001	4
181		15	max	307.879	2	-.162	15	.018	1	0	2	0	2	15
182		min	-257.036	3	-.694	4	-.002	3	0	3	0	3	0	4
183		16	max	307.811	2	-.204	15	.018	1	0	2	0	2	15
184		min	-257.087	3	-.871	4	-.002	3	0	3	0	3	0	4
185		17	max	307.744	2	-.246	15	.018	1	0	2	0	2	15
186		min	-257.138	3	-1.049	4	-.002	3	0	3	0	3	0	4
187		18	max	307.676	2	-.287	15	.018	1	0	2	0	2	15
188		min	-257.189	3	-1.227	4	-.002	3	0	3	0	3	0	4
189		19	max	307.608	2	-.329	15	.018	1	0	2	0	2	1
190		min	-257.24	3	-1.404	4	-.002	3	0	3	0	3	0	1
191	M8	1	max	1602.049	1	0	1	.833	1	0	1	0	15	0
192		min	-242.797	3	0	1	-.233	3	0	1	0	1	0	1
193		2	max	1602.114	1	0	1	.833	1	0	1	0	1	0
194		min	-242.749	3	0	1	-.233	3	0	1	0	3	0	1
195		3	max	1602.179	1	0	1	.833	1	0	1	0	1	0
196		min	-242.7	3	0	1	-.233	3	0	1	0	3	0	1
197		4	max	1602.244	1	0	1	.833	1	0	1	0	1	0
198		min	-242.652	3	0	1	-.233	3	0	1	0	3	0	1
199		5	max	1602.308	1	0	1	.833	1	0	1	0	1	0
200		min	-242.603	3	0	1	-.233	3	0	1	0	3	0	1
201		6	max	1602.373	1	0	1	.833	1	0	1	0	1	0
202		min	-242.555	3	0	1	-.233	3	0	1	0	3	0	1
203		7	max	1602.438	1	0	1	.833	1	0	1	0	1	0
204		min	-242.506	3	0	1	-.233	3	0	1	0	3	0	1
205		8	max	1602.502	1	0	1	.833	1	0	1	0	1	0
206		min	-242.458	3	0	1	-.233	3	0	1	0	3	0	1
207		9	max	1602.567	1	0	1	.833	1	0	1	0	1	0
208		min	-242.409	3	0	1	-.233	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1602.632	1	0	1	.833	1	0	1	0	1	0	1
210			min	-242.361	3	0	1	-.233	3	0	1	0	3	0	1
211		11	max	1602.697	1	0	1	.833	1	0	1	0	1	0	1
212			min	-242.312	3	0	1	-.233	3	0	1	0	3	0	1
213		12	max	1602.761	1	0	1	.833	1	0	1	0	1	0	1
214			min	-242.264	3	0	1	-.233	3	0	1	0	3	0	1
215		13	max	1602.826	1	0	1	.833	1	0	1	0	1	0	1
216			min	-242.215	3	0	1	-.233	3	0	1	0	3	0	1
217		14	max	1602.891	1	0	1	.833	1	0	1	0	1	0	1
218			min	-242.166	3	0	1	-.233	3	0	1	0	3	0	1
219		15	max	1602.955	1	0	1	.833	1	0	1	.001	1	0	1
220			min	-242.118	3	0	1	-.233	3	0	1	0	3	0	1
221		16	max	1603.02	1	0	1	.833	1	0	1	.001	1	0	1
222			min	-242.069	3	0	1	-.233	3	0	1	0	3	0	1
223		17	max	1603.085	1	0	1	.833	1	0	1	.001	1	0	1
224			min	-242.021	3	0	1	-.233	3	0	1	0	3	0	1
225		18	max	1603.15	1	0	1	.833	1	0	1	.001	1	0	1
226			min	-241.972	3	0	1	-.233	3	0	1	0	3	0	1
227		19	max	1603.214	1	0	1	.833	1	0	1	.001	1	0	1
228			min	-241.924	3	0	1	-.233	3	0	1	0	3	0	1
229	M10	1	max	413.185	1	.633	4	-.007	15	.001	1	0	1	0	1
230			min	-338.084	3	.151	15	-.191	1	0	3	0	3	0	1
231		2	max	413.291	1	.591	4	-.007	15	.001	1	0	1	0	15
232			min	-338.004	3	.141	15	-.191	1	0	3	0	3	0	4
233		3	max	413.398	1	.55	4	-.007	15	.001	1	0	1	0	15
234			min	-337.924	3	.131	15	-.191	1	0	3	0	3	0	4
235		4	max	413.504	1	.509	4	-.007	15	.001	1	0	1	0	15
236			min	-337.844	3	.122	15	-.191	1	0	3	0	3	0	4
237		5	max	413.611	1	.467	4	-.007	15	.001	1	0	2	0	15
238			min	-337.764	3	.112	15	-.191	1	0	3	0	3	0	4
239		6	max	413.718	1	.426	4	-.007	15	.001	1	0	2	0	15
240			min	-337.685	3	.102	15	-.191	1	0	3	0	3	0	4
241		7	max	413.824	1	.385	4	-.007	15	.001	1	0	2	0	15
242			min	-337.605	3	.093	15	-.191	1	0	3	0	3	0	4
243		8	max	413.931	1	.344	4	-.007	15	.001	1	0	2	0	15
244			min	-337.525	3	.083	15	-.191	1	0	3	0	3	0	4
245		9	max	414.037	1	.302	4	-.007	15	.001	1	0	2	0	15
246			min	-337.445	3	.073	15	-.191	1	0	3	0	1	0	4
247		10	max	414.144	1	.261	4	-.007	15	.001	1	0	15	0	15
248			min	-337.365	3	.063	15	-.191	1	0	3	0	1	0	4
249		11	max	414.25	1	.22	4	-.007	15	.001	1	0	15	0	15
250			min	-337.285	3	.054	15	-.191	1	0	3	0	1	0	4
251		12	max	414.357	1	.179	4	-.007	15	.001	1	0	15	0	15
252			min	-337.205	3	.044	15	-.191	1	0	3	0	1	0	4
253		13	max	414.463	1	.137	4	-.007	15	.001	1	0	15	0	15
254			min	-337.125	3	.034	15	-.191	1	0	3	0	1	0	4
255		14	max	414.57	1	.096	4	-.007	15	.001	1	0	15	0	15
256			min	-337.045	3	.013	1	-.191	1	0	3	0	1	0	4
257		15	max	414.676	1	.062	10	-.007	15	.001	1	0	15	0	15
258			min	-336.965	3	-.019	1	-.191	1	0	3	0	1	0	4
259		16	max	414.783	1	.036	10	-.007	15	.001	1	0	15	0	15
260			min	-336.885	3	-.051	1	-.191	1	0	3	0	1	0	4
261		17	max	414.889	1	.009	10	-.007	15	.001	1	0	15	0	15
262			min	-336.806	3	-.083	1	-.191	1	0	3	0	1	0	4
263		18	max	414.996	1	-.014	15	-.007	15	.001	1	0	15	0	15
264			min	-336.726	3	-.115	1	-.191	1	0	3	0	1	0	4
265		19	max	415.103	1	-.024	15	-.007	15	.001	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-336.646	3	-.148	1	-.191	1	0	3	0	1	0	4
267			max	58.42	10	1.8	4	.98	1	.001	1	0	3	0	4
268			min	-108.584	9	.423	15	.023	12	0	15	-.002	1	0	15
269		2	max	58.364	10	1.622	4	.98	1	.001	1	0	3	0	2
270			min	-108.64	9	.382	15	.023	12	0	15	-.002	1	0	12
271		3	max	58.307	10	1.444	4	.98	1	.001	1	0	3	0	2
272			min	-108.697	9	.34	15	.023	12	0	15	-.002	1	0	3
273		4	max	58.251	10	1.267	4	.98	1	.001	1	0	3	0	15
274			min	-108.753	9	.298	15	.023	12	0	15	-.002	1	0	3
275		5	max	58.194	10	1.089	4	.98	1	.001	1	0	3	0	15
276			min	-108.81	9	.256	15	.023	12	0	15	-.002	1	0	4
277		6	max	58.138	10	.911	4	.98	1	.001	1	0	3	0	15
278			min	-108.867	9	.215	15	.023	12	0	15	-.001	1	0	4
279		7	max	58.081	10	.734	4	.98	1	.001	1	0	3	0	15
280			min	-108.923	9	.173	15	.023	12	0	15	-.001	1	0	4
281		8	max	58.025	10	.556	4	.98	1	.001	1	0	3	0	15
282			min	-108.98	9	.131	15	.023	12	0	15	-.001	1	0	4
283		9	max	57.968	10	.378	4	.98	1	.001	1	0	3	0	15
284	min		-109.036	9	.089	15	.023	12	0	15	0	1	-.001	4	
285	10	max	57.911	10	.201	4	.98	1	.001	1	0	3	0	15	
286		min	-109.093	9	.048	15	.023	12	0	15	0	1	-.001	4	
287	11	max	57.855	10	.048	2	.98	1	.001	1	0	3	0	15	
288		min	-109.149	9	-.019	3	.023	12	0	15	0	1	-.001	4	
289	12	max	57.798	10	-.036	15	.98	1	.001	1	0	3	0	15	
290		min	-109.206	9	-.155	4	.023	12	0	15	0	1	-.001	4	
291	13	max	57.742	10	-.078	15	.98	1	.001	1	0	3	0	15	
292		min	-109.262	9	-.332	4	.023	12	0	15	0	2	-.001	4	
293	14	max	57.685	10	-.12	15	.98	1	.001	1	0	1	0	15	
294		min	-109.319	9	-.51	4	.023	12	0	15	0	10	-.001	4	
295	15	max	57.629	10	-.161	15	.98	1	.001	1	0	1	0	15	
296		min	-109.375	9	-.688	4	.023	12	0	15	0	15	0	4	
297	16	max	57.572	10	-.203	15	.98	1	.001	1	0	1	0	15	
298		min	-109.432	9	-.865	4	.023	12	0	15	0	15	0	4	
299	17	max	57.516	10	-.245	15	.98	1	.001	1	0	1	0	15	
300		min	-109.489	9	-1.043	4	.023	12	0	15	0	15	0	4	
301	18	max	57.459	10	-.287	15	.98	1	.001	1	.001	1	0	15	
302		min	-109.545	9	-1.22	4	.023	12	0	15	0	15	0	4	
303	19	max	57.402	10	-.328	15	.98	1	.001	1	.001	1	0	1	
304		min	-109.602	9	-1.398	4	.023	12	0	15	0	15	0	1	
305	M12	1	max	561.611	1	0	1	4.521	1	0	1	0	1	0	1
306			min	-65.338	3	0	1	.15	15	0	1	0	3	0	1
307		2	max	561.676	1	0	1	4.521	1	0	1	0	1	0	1
308			min	-65.29	3	0	1	.15	15	0	1	0	15	0	1
309		3	max	561.741	1	0	1	4.521	1	0	1	0	1	0	1
310			min	-65.241	3	0	1	.15	15	0	1	0	15	0	1
311		4	max	561.805	1	0	1	4.521	1	0	1	.001	1	0	1
312			min	-65.193	3	0	1	.15	15	0	1	0	15	0	1
313		5	max	561.87	1	0	1	4.521	1	0	1	.002	1	0	1
314			min	-65.144	3	0	1	.15	15	0	1	0	15	0	1
315		6	max	561.935	1	0	1	4.521	1	0	1	.002	1	0	1
316			min	-65.096	3	0	1	.15	15	0	1	0	15	0	1
317		7	max	561.999	1	0	1	4.521	1	0	1	.002	1	0	1
318			min	-65.047	3	0	1	.15	15	0	1	0	15	0	1
319		8	max	562.064	1	0	1	4.521	1	0	1	.003	1	0	1
320			min	-64.999	3	0	1	.15	15	0	1	0	15	0	1
321		9	max	562.129	1	0	1	4.521	1	0	1	.003	1	0	1
322			min	-64.95	3	0	1	.15	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	562.193	1	0	1	4.521	1	0	1	.004	1	0	1
324		min	-64.902	3	0	1	.15	15	0	1	0	15	0	1
325	11	max	562.258	1	0	1	4.521	1	0	1	.004	1	0	1
326		min	-64.853	3	0	1	.15	15	0	1	0	15	0	1
327	12	max	562.323	1	0	1	4.521	1	0	1	.004	1	0	1
328		min	-64.805	3	0	1	.15	15	0	1	0	15	0	1
329	13	max	562.388	1	0	1	4.521	1	0	1	.005	1	0	1
330		min	-64.756	3	0	1	.15	15	0	1	0	15	0	1
331	14	max	562.452	1	0	1	4.521	1	0	1	.005	1	0	1
332		min	-64.707	3	0	1	.15	15	0	1	0	15	0	1
333	15	max	562.517	1	0	1	4.521	1	0	1	.006	1	0	1
334		min	-64.659	3	0	1	.15	15	0	1	0	15	0	1
335	16	max	562.582	1	0	1	4.521	1	0	1	.006	1	0	1
336		min	-64.61	3	0	1	.15	15	0	1	0	15	0	1
337	17	max	562.646	1	0	1	4.521	1	0	1	.007	1	0	1
338		min	-64.562	3	0	1	.15	15	0	1	0	15	0	1
339	18	max	562.711	1	0	1	4.521	1	0	1	.007	1	0	1
340		min	-64.513	3	0	1	.15	15	0	1	0	15	0	1
341	19	max	562.776	1	0	1	4.521	1	0	1	.007	1	0	1
342		min	-64.465	3	0	1	.15	15	0	1	0	15	0	1
343	M1	1	max	143.833	1	328.569	3	-2.989	15	0	.175	1	.014	1
344		min	4.803	15	-397.376	1	-88.688	1	0	3	.006	15	-.009	3
345	2	max	143.929	1	328.372	3	-2.989	15	0	1	.156	1	.1	1
346		min	4.831	15	-397.639	1	-88.688	1	0	3	.005	15	-.081	3
347	3	max	122.157	1	7.599	9	-2.961	15	0	12	.135	1	.184	1
348		min	4.372	15	-20.39	3	-88.417	1	0	1	.005	15	-.151	3
349	4	max	122.253	1	7.38	9	-2.961	15	0	12	.116	1	.184	1
350		min	4.401	15	-20.587	3	-88.417	1	0	1	.004	15	-.146	3
351	5	max	122.348	1	7.161	9	-2.961	15	0	12	.097	1	.184	1
352		min	4.429	15	-20.784	3	-88.417	1	0	1	.003	15	-.142	3
353	6	max	122.444	1	6.943	9	-2.961	15	0	12	.078	1	.184	1
354		min	4.458	15	-20.981	3	-88.417	1	0	1	.003	15	-.137	3
355	7	max	122.539	1	6.724	9	-2.961	15	0	12	.059	1	.184	1
356		min	4.487	15	-21.177	3	-88.417	1	0	1	.002	15	-.133	3
357	8	max	122.635	1	6.505	9	-2.961	15	0	12	.039	1	.184	1
358		min	4.516	15	-21.374	3	-88.417	1	0	1	.001	15	-.128	3
359	9	max	122.73	1	6.287	9	-2.961	15	0	12	.02	1	.184	1
360		min	4.545	15	-21.571	3	-88.417	1	0	1	0	15	-.123	3
361	10	max	122.826	1	6.068	9	-2.961	15	0	12	.001	1	.185	1
362		min	4.574	15	-21.768	3	-88.417	1	0	1	0	10	-.119	3
363	11	max	122.921	1	5.849	9	-2.961	15	0	12	0	12	.185	1
364		min	4.602	15	-21.965	3	-88.417	1	0	1	-.018	1	-.114	3
365	12	max	123.017	1	5.631	9	-2.961	15	0	12	-.001	12	.185	1
366		min	4.631	15	-22.161	3	-88.417	1	0	1	-.037	1	-.109	3
367	13	max	123.112	1	5.412	9	-2.961	15	0	12	-.002	12	.186	1
368		min	4.66	15	-22.358	3	-88.417	1	0	1	-.056	1	-.104	3
369	14	max	123.208	1	5.193	9	-2.961	15	0	12	-.003	15	.186	1
370		min	4.689	15	-22.555	3	-88.417	1	0	1	-.076	1	-.099	3
371	15	max	123.303	1	4.975	9	-2.961	15	0	12	-.003	15	.186	1
372		min	4.718	15	-22.752	3	-88.417	1	0	1	-.095	1	-.094	3
373	16	max	79.479	2	27.826	10	-2.989	15	0	1	-.004	15	.188	1
374		min	-29.889	3	-84.878	3	-89.155	1	0	12	-.115	1	-.089	3
375	17	max	79.575	2	27.608	10	-2.989	15	0	1	-.004	15	.205	1
376		min	-29.818	3	-85.075	3	-89.155	1	0	12	-.134	1	-.07	3
377	18	max	-4.808	15	444.903	1	-3.062	15	0	3	-.005	15	.111	1
378		min	-143.283	1	-147.152	3	-91.355	1	0	1	-.154	1	-.039	3
379	19	max	-4.779	15	444.64	1	-3.062	15	0	3	-.006	15	.015	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-143.188	1	-147.348	3	-91.355	1	0	1	-.174	1	-.007	3
381	M5	max	314.34	1	1086.7	3	-.109	10	0	1	.006	1	.019	3
382		min	9.416	12	-1315.435	1	-32.243	1	0	3	0	10	-.027	1
383		max	314.436	1	1086.503	3	-.109	10	0	1	0	2	.258	1
384		min	9.464	12	-1315.697	1	-32.243	1	0	3	-.003	3	-.217	3
385		max	242.463	1	9.388	9	2.934	3	0	3	0	10	.538	1
386		min	7.679	10	-67.793	3	-.091	10	0	1	-.008	3	-.447	3
387		max	242.559	1	9.169	9	2.934	3	0	3	0	10	.542	1
388		min	7.758	10	-67.99	3	-.091	10	0	1	-.007	1	-.433	3
389		max	242.654	1	8.951	9	2.934	3	0	3	0	10	.546	1
390		min	7.838	10	-68.187	3	-.091	10	0	1	-.007	1	-.418	3
391		max	242.75	1	8.732	9	2.934	3	0	3	0	10	.55	1
392		min	7.918	10	-68.384	3	-.091	10	0	1	-.006	1	-.403	3
393		max	242.845	1	8.513	9	2.934	3	0	3	0	10	.554	1
394		min	7.997	10	-68.58	3	-.091	10	0	1	-.006	1	-.388	3
395		max	242.941	1	8.295	9	2.934	3	0	3	0	10	.559	1
396		min	8.077	10	-68.777	3	-.091	10	0	1	-.005	1	-.373	3
397		max	243.036	1	8.076	9	2.934	3	0	3	0	10	.563	1
398		min	8.156	10	-68.974	3	-.091	10	0	1	-.005	1	-.358	3
399		max	243.132	1	7.857	9	2.934	3	0	3	0	10	.567	1
400		min	8.236	10	-69.171	3	-.091	10	0	1	-.004	1	-.344	3
401		max	243.227	1	7.639	9	2.934	3	0	3	0	10	.572	1
402		min	8.316	10	-69.368	3	-.091	10	0	1	-.004	1	-.328	3
403		max	243.323	1	7.42	9	2.934	3	0	3	0	10	.576	1
404		min	8.395	10	-69.564	3	-.091	10	0	1	-.003	1	-.313	3
405		max	243.418	1	7.201	9	2.934	3	0	3	0	10	.581	1
406		min	8.475	10	-69.761	3	-.091	10	0	1	-.003	1	-.298	3
407		max	243.514	1	6.983	9	2.934	3	0	3	0	15	.585	1
408		min	8.554	10	-69.958	3	-.091	10	0	1	-.002	1	-.283	3
409		max	243.609	1	6.764	9	2.934	3	0	3	0	15	.59	1
410		min	8.634	10	-70.155	3	-.091	10	0	1	-.002	1	-.268	3
411		max	288.223	2	162.397	2	2.911	3	0	1	0	3	.595	1
412		min	-97.794	3	-255.636	3	-.095	10	0	15	-.001	1	-.251	3
413		max	288.318	2	162.135	2	2.911	3	0	1	0	3	.601	1
414		min	-97.722	3	-255.833	3	-.095	10	0	15	0	1	-.196	3
415		max	-9.909	12	1466.372	1	2.938	1	0	3	.001	3	.289	1
416		min	-315.301	1	-484.841	3	-.019	10	0	1	0	2	-.091	3
417		max	-9.862	12	1466.109	1	2.938	1	0	3	.002	3	.014	3
418		min	-315.205	1	-485.038	3	-.019	10	0	1	0	2	-.029	1
419	M9	max	143.153	1	328.557	3	120.87	1	0	3	-.006	15	.014	1
420		min	4.778	15	-397.354	1	4.183	15	0	1	-.175	1	-.009	3
421		max	143.249	1	328.361	3	120.87	1	0	3	-.004	12	.1	1
422		min	4.807	15	-397.616	1	4.183	15	0	1	-.149	1	-.081	3
423		max	122.112	1	7.573	9	83.042	1	0	1	0	3	.184	1
424		min	4.556	15	-20.335	3	1.618	12	0	15	-.121	1	-.15	3
425		max	122.208	1	7.355	9	83.042	1	0	1	.001	3	.184	1
426		min	4.585	15	-20.532	3	1.618	12	0	15	-.103	1	-.146	3
427		max	122.303	1	7.136	9	83.042	1	0	1	.002	3	.184	1
428		min	4.614	15	-20.729	3	1.618	12	0	15	-.085	1	-.142	3
429		max	122.399	1	6.917	9	83.042	1	0	1	.002	3	.184	1
430		min	4.642	15	-20.926	3	1.618	12	0	15	-.067	1	-.137	3
431		max	122.494	1	6.699	9	83.042	1	0	1	.003	3	.184	1
432		min	4.671	15	-21.122	3	1.618	12	0	15	-.049	1	-.132	3
433		max	122.59	1	6.48	9	83.042	1	0	1	.003	3	.184	1
434		min	4.7	15	-21.319	3	1.618	12	0	15	-.031	1	-.128	3
435		max	122.685	1	6.261	9	83.042	1	0	1	.004	3	.184	1
436		min	4.729	15	-21.516	3	1.618	12	0	15	-.013	1	-.123	3





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	2.989	15	-396.695	1	4.803	15	-.014	1	.006	15	0	3
495	M16	1	max	-1.884	12	445.339	1	-4.771	15	.007	3	.172	1	0	1
496			min	-88.094	1	-147.367	3	-142.947	1	-.015	1	.006	15	0	3
497		2	max	-1.884	12	314.115	1	-3.657	15	.007	3	.052	1	.119	3
498			min	-88.094	1	-104.04	3	-109.54	1	-.015	1	.002	15	-.359	1
499		3	max	-1.884	12	182.892	1	-2.544	15	.007	3	-.001	12	.197	3
500			min	-88.094	1	-60.712	3	-76.133	1	-.015	1	-.035	1	-.593	1
501		4	max	-1.884	12	51.668	1	-1.431	15	.007	3	-.003	15	.233	3
502			min	-88.094	1	-17.385	3	-42.726	1	-.015	1	-.092	1	-.704	1
503		5	max	-1.884	12	25.943	3	-.318	15	.007	3	-.004	15	.229	3
504			min	-88.094	1	-79.556	1	-9.319	1	-.015	1	-.116	1	-.691	1
505		6	max	-1.884	12	69.27	3	24.088	1	.007	3	-.004	15	.184	3
506			min	-88.094	1	-210.78	1	.529	12	-.015	1	-.109	1	-.554	1
507		7	max	-1.884	12	112.598	3	57.495	1	.007	3	-.002	15	.099	3
508			min	-88.094	1	-342.003	1	1.615	12	-.015	1	-.071	1	-.293	1
509		8	max	-1.884	12	155.925	3	90.902	1	.007	3	0	2	.092	1
510			min	-88.094	1	-473.227	1	2.701	12	-.015	1	-.002	3	-.028	3
511		9	max	-1.884	12	199.252	3	124.309	1	.007	3	.101	1	.601	1
512			min	-88.094	1	-604.451	1	3.788	12	-.015	1	.002	12	-.196	3
513		10	max	-3.061	15	-17.148	15	157.716	1	0	15	.235	1	1.234	1
514			min	-91.059	1	-735.674	1	-7.47	3	-.015	1	.007	12	-.405	3
515		11	max	-3.061	15	604.451	1	-3.902	12	.015	1	.102	1	.601	1
516			min	-91.059	1	-199.252	3	-124.055	1	-.007	3	.003	12	-.196	3
517		12	max	-3.061	15	473.227	1	-2.816	12	.015	1	0	2	.092	1
518			min	-91.059	1	-155.925	3	-90.648	1	-.007	3	0	3	-.028	3
519		13	max	-3.061	15	342.003	1	-1.729	12	.015	1	-.002	12	.099	3
520			min	-91.059	1	-112.598	3	-57.241	1	-.007	3	-.07	1	-.293	1
521		14	max	-3.061	15	210.779	1	-.643	12	.015	1	-.003	12	.184	3
522			min	-91.059	1	-69.27	3	-23.833	1	-.007	3	-.108	1	-.554	1
523		15	max	-3.061	15	79.556	1	9.574	1	.015	1	-.004	12	.229	3
524			min	-91.059	1	-25.943	3	.326	15	-.007	3	-.115	1	-.691	1
525		16	max	-3.061	15	17.385	3	42.981	1	.015	1	-.003	12	.233	3
526			min	-91.059	1	-51.668	1	1.439	15	-.007	3	-.09	1	-.704	1
527		17	max	-3.061	15	60.712	3	76.388	1	.015	1	0	12	.197	3
528			min	-91.059	1	-182.892	1	2.553	15	-.007	3	-.033	1	-.593	1
529		18	max	-3.061	15	104.04	3	109.795	1	.015	1	.055	1	.119	3
530			min	-91.059	1	-314.115	1	3.666	15	-.007	3	.002	15	-.359	1
531		19	max	-3.061	15	147.367	3	143.202	1	.015	1	.174	1	0	1
532			min	-91.059	1	-445.339	1	4.779	15	-.007	3	.006	15	0	3
533	M15	1	max	0	10	2.958	4	.021	3	0	1	0	1	0	1
534			min	-35.432	1	0	10	-.028	1	0	3	0	3	0	1
535		2	max	0	10	2.63	4	.021	3	0	1	0	1	0	10
536			min	-35.511	1	0	10	-.028	1	0	3	0	3	-.001	4
537		3	max	0	10	2.301	4	.021	3	0	1	0	1	0	10
538			min	-35.591	1	0	10	-.028	1	0	3	0	3	-.003	4
539		4	max	0	10	1.972	4	.021	3	0	1	0	1	0	10
540			min	-35.67	1	0	10	-.028	1	0	3	0	3	-.004	4
541		5	max	0	10	1.643	4	.021	3	0	1	0	1	0	10
542			min	-35.75	1	0	10	-.028	1	0	3	0	3	-.005	4
543		6	max	0	10	1.315	4	.021	3	0	1	0	1	0	10
544			min	-35.829	1	0	10	-.028	1	0	3	0	3	-.005	4
545		7	max	0	10	.986	4	.021	3	0	1	0	3	0	10
546			min	-35.909	1	0	10	-.028	1	0	3	0	2	-.006	4
547		8	max	0	10	.657	4	.021	3	0	1	0	3	0	10
548			min	-35.988	1	0	10	-.028	1	0	3	0	1	-.006	4
549		9	max	0	10	.329	4	.021	3	0	1	0	3	0	10
550			min	-36.068	1	0	10	-.028	1	0	3	0	1	-.007	4

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	10	0	1	.021	3	0	1	0	3	0	10
552			min	-36.148	1	0	1	-.028	1	0	3	0	1	-.007	4
553		11	max	0	10	0	10	.021	3	0	1	0	3	0	10
554			min	-36.227	1	-.329	4	-.028	1	0	3	0	1	-.007	4
555		12	max	0	10	0	10	.021	3	0	1	0	3	0	10
556			min	-36.307	1	-.657	4	-.028	1	0	3	0	1	-.006	4
557		13	max	0	10	0	10	.021	3	0	1	0	3	0	10
558			min	-36.386	1	-.986	4	-.028	1	0	3	0	1	-.006	4
559		14	max	0	10	0	10	.021	3	0	1	0	3	0	10
560			min	-36.466	1	-1.315	4	-.028	1	0	3	0	1	-.005	4
561		15	max	0	10	0	10	.021	3	0	1	0	3	0	10
562			min	-36.545	1	-1.643	4	-.028	1	0	3	0	1	-.005	4
563		16	max	0	10	0	10	.021	3	0	1	0	3	0	10
564			min	-36.625	1	-1.972	4	-.028	1	0	3	0	1	-.004	4
565		17	max	0	10	0	10	.021	3	0	1	0	3	0	10
566			min	-36.704	1	-2.301	4	-.028	1	0	3	0	1	-.003	4
567		18	max	0	10	0	10	.021	3	0	1	0	3	0	10
568			min	-36.784	1	-2.63	4	-.028	1	0	3	0	1	-.001	4
569		19	max	0	10	0	10	.021	3	0	1	0	3	0	1
570			min	-36.864	1	-2.958	4	-.028	1	0	3	0	1	0	1
571	M16A	1	max	-.918	10	2.958	4	.02	1	0	3	0	3	0	1
572			min	-40.693	1	.695	15	-.009	3	0	1	0	1	0	1
573		2	max	-.852	10	2.63	4	.02	1	0	3	0	3	0	15
574			min	-40.614	1	.618	15	-.009	3	0	1	0	1	-.001	4
575		3	max	-.786	10	2.301	4	.02	1	0	3	0	3	0	15
576			min	-40.534	1	.541	15	-.009	3	0	1	0	1	-.003	4
577		4	max	-.719	10	1.972	4	.02	1	0	3	0	3	0	15
578			min	-40.455	1	.464	15	-.009	3	0	1	0	1	-.004	4
579		5	max	-.653	10	1.643	4	.02	1	0	3	0	3	-.001	15
580			min	-40.375	1	.386	15	-.009	3	0	1	0	1	-.005	4
581		6	max	-.587	10	1.315	4	.02	1	0	3	0	3	-.001	15
582			min	-40.296	1	.309	15	-.009	3	0	1	0	1	-.005	4
583		7	max	-.52	10	.986	4	.02	1	0	3	0	3	-.001	15
584			min	-40.216	1	.232	15	-.009	3	0	1	0	1	-.006	4
585		8	max	-.454	10	.657	4	.02	1	0	3	0	3	-.001	15
586			min	-40.136	1	.155	15	-.009	3	0	1	0	1	-.006	4
587		9	max	-.388	10	.329	4	.02	1	0	3	0	3	-.002	15
588			min	-40.057	1	.077	15	-.009	3	0	1	0	1	-.007	4
589		10	max	-.322	10	0	1	.02	1	0	3	0	3	-.002	15
590			min	-39.977	1	0	1	-.009	3	0	1	0	1	-.007	4
591		11	max	-.255	10	-.077	15	.02	1	0	3	0	3	-.002	15
592			min	-39.898	1	-.329	4	-.009	3	0	1	0	1	-.007	4
593		12	max	-.189	10	-.155	15	.02	1	0	3	0	3	-.001	15
594			min	-39.818	1	-.657	4	-.009	3	0	1	0	1	-.006	4
595		13	max	-.123	10	-.232	15	.02	1	0	3	0	1	-.001	15
596			min	-39.739	1	-.986	4	-.009	3	0	1	0	13	-.006	4
597		14	max	-.056	10	-.309	15	.02	1	0	3	0	1	-.001	15
598			min	-39.659	1	-1.315	4	-.009	3	0	1	0	3	-.005	4
599		15	max	.01	10	-.386	15	.02	1	0	3	0	1	-.001	15
600			min	-39.58	1	-1.643	4	-.009	3	0	1	0	3	-.005	4
601		16	max	.076	10	-.464	15	.02	1	0	3	0	1	0	15
602			min	-39.5	1	-1.972	4	-.009	3	0	1	0	3	-.004	4
603		17	max	.142	10	-.541	15	.02	1	0	3	0	1	0	15
604			min	-39.42	1	-2.301	4	-.009	3	0	1	0	3	-.003	4
605		18	max	.209	10	-.618	15	.02	1	0	3	0	1	0	15
606			min	-39.341	1	-2.63	4	-.009	3	0	1	0	3	-.001	4
607		19	max	.275	10	-.695	15	.02	1	0	3	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-39.261	1	-2.958	4	-.009	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.007	2	.016	1	-4.506e-5	15	NC	3	NC	3	
2			min	-.003	3	-.006	3	0	3	-1.346e-3	1	4574.669	2	2024.261	1	
3			2	max	.003	1	.007	2	.015	1	-4.32e-5	15	NC	3	NC	3
4				min	-.003	3	-.006	3	0	3	-1.291e-3	1	4962.582	2	2190.534	1
5			3	max	.003	1	.006	2	.014	1	-4.133e-5	15	NC	3	NC	3
6				min	-.003	3	-.006	3	0	3	-1.235e-3	1	5418.769	2	2386.31	1
7			4	max	.003	1	.006	2	.013	1	-3.947e-5	15	NC	3	NC	3
8				min	-.002	3	-.005	3	0	3	-1.18e-3	1	5958.844	2	2618.742	1
9			5	max	.003	1	.005	2	.011	1	-3.76e-5	15	NC	3	NC	3
10				min	-.002	3	-.005	3	0	3	-1.125e-3	1	6603.371	2	2897.373	1
11		6	max	.002	1	.005	2	.01	1	-3.573e-5	15	NC	1	NC	3	
12			min	-.002	3	-.005	3	0	3	-1.069e-3	1	7379.877	2	3235.17	1	
13		7	max	.002	1	.004	2	.009	1	-3.387e-5	15	NC	1	NC	3	
14			min	-.002	3	-.005	3	0	3	-1.014e-3	1	8325.9	2	3650.121	1	
15		8	max	.002	1	.004	2	.008	1	-3.2e-5	15	NC	1	NC	2	
16			min	-.002	3	-.004	3	0	3	-9.587e-4	1	9493.755	2	4167.779	1	
17		9	max	.002	1	.003	2	.007	1	-3.013e-5	15	NC	1	NC	2	
18			min	-.002	3	-.004	3	0	3	-9.034e-4	1	NC	1	4825.48	1	
19		10	max	.002	1	.003	2	.006	1	-2.827e-5	15	NC	1	NC	2	
20			min	-.001	3	-.004	3	0	3	-8.481e-4	1	NC	1	5679.599	1	
21		11	max	.001	1	.002	2	.005	1	-2.64e-5	15	NC	1	NC	2	
22			min	-.001	3	-.003	3	0	3	-7.927e-4	1	NC	1	6818.688	1	
23		12	max	.001	1	.002	2	.004	1	-2.454e-5	15	NC	1	NC	2	
24			min	-.001	3	-.003	3	0	3	-7.374e-4	1	NC	1	8388.732	1	
25		13	max	.001	1	.001	2	.003	1	-2.267e-5	15	NC	1	NC	1	
26			min	0	3	-.003	3	0	3	-6.821e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-2.08e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-6.267e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-1.894e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-5.714e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-1.707e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	12	-5.161e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.52e-5	15	NC	1	NC	1	
34			min	0	3	0	3	0	12	-4.607e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.334e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-4.054e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-9.848e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.501e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.609e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	4.629e-6	12	NC	1	NC	1	
41			2	max	0	9	0	2	0	12	2.032e-4	1	NC	1	NC	1
42				min	0	10	0	3	0	1	6.705e-6	15	NC	1	NC	1
43			3	max	0	9	0	2	0	12	2.456e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	-.001	1	8.134e-6	15	NC	1	NC	1
45			4	max	0	9	0	2	0	12	2.879e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	-.001	1	9.564e-6	15	NC	1	NC	1
47			5	max	0	9	0	2	0	12	3.302e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	-.001	1	1.099e-5	15	NC	1	NC	1
49			6	max	0	9	0	2	0	3	3.725e-4	1	NC	1	NC	1
50				min	0	10	-.004	3	-.001	1	1.242e-5	15	NC	1	NC	1
51		7	max	0	9	0	2	0	3	4.148e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	10	-.004	3	-.001	1	1.385e-5	15	NC	1	NC	1
53		8	max	0	9	.001	2	0	3	4.571e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	1.528e-5	15	NC	1	NC	1
55		9	max	0	9	.002	2	0	3	4.995e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	2	1.671e-5	15	NC	1	NC	1
57		10	max	0	9	.002	2	0	1	5.418e-4	1	NC	1	NC	1
58			min	0	10	-.006	3	0	15	1.814e-5	15	NC	1	NC	1
59		11	max	0	9	.003	2	.001	1	5.841e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	15	1.957e-5	15	NC	1	NC	1
61		12	max	0	9	.003	2	.002	1	6.264e-4	1	NC	1	NC	1
62			min	0	10	-.007	3	0	15	2.1e-5	15	NC	1	NC	1
63		13	max	0	9	.004	2	.003	1	6.687e-4	1	NC	1	NC	1
64			min	0	10	-.007	3	0	15	2.243e-5	15	NC	1	NC	1
65		14	max	0	9	.005	2	.004	1	7.111e-4	1	NC	1	NC	1
66			min	0	10	-.007	3	0	15	2.386e-5	15	9980.391	2	NC	1
67		15	max	0	9	.005	2	.005	1	7.534e-4	1	NC	3	NC	2
68			min	0	10	-.007	3	0	15	2.529e-5	15	8396.026	2	9005.239	1
69		16	max	.001	9	.006	2	.006	1	7.957e-4	1	NC	3	NC	2
70			min	0	10	-.007	3	0	15	2.672e-5	15	7167.701	2	7573.807	1
71		17	max	.001	9	.007	2	.007	1	8.38e-4	1	NC	3	NC	2
72			min	0	10	-.007	3	0	15	2.815e-5	15	6205.429	2	6551.439	1
73		18	max	.001	9	.008	2	.008	1	8.803e-4	1	NC	3	NC	2
74			min	0	10	-.007	3	0	15	2.958e-5	15	5444.762	2	5798.417	1
75		19	max	.001	9	.01	2	.009	1	9.226e-4	1	NC	3	NC	2
76			min	0	10	-.007	3	0	15	3.101e-5	15	4839.202	2	5232.762	1
77	M4	1	max	.003	1	.008	2	0	15	-3.704e-5	15	NC	1	NC	3
78			min	0	3	-.006	3	-.007	1	-1.115e-3	1	NC	1	2924.672	1
79		2	max	.003	1	.008	2	0	15	-3.704e-5	15	NC	1	NC	3
80			min	0	3	-.006	3	-.006	1	-1.115e-3	1	NC	1	3190.185	1
81		3	max	.002	1	.008	2	0	15	-3.704e-5	15	NC	1	NC	3
82			min	0	3	-.006	3	-.006	1	-1.115e-3	1	NC	1	3506.204	1
83		4	max	.002	1	.007	2	0	15	-3.704e-5	15	NC	1	NC	2
84			min	0	3	-.005	3	-.005	1	-1.115e-3	1	NC	1	3886.05	1
85		5	max	.002	1	.007	2	0	15	-3.704e-5	15	NC	1	NC	2
86			min	0	3	-.005	3	-.004	1	-1.115e-3	1	NC	1	4347.866	1
87		6	max	.002	1	.006	2	0	15	-3.704e-5	15	NC	1	NC	2
88			min	0	3	-.005	3	-.004	1	-1.115e-3	1	NC	1	4916.888	1
89		7	max	.002	1	.006	2	0	15	-3.704e-5	15	NC	1	NC	2
90			min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	5629.065	1
91		8	max	.002	1	.005	2	0	15	-3.704e-5	15	NC	1	NC	2
92			min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	6537.039	1
93		9	max	.001	1	.005	2	0	15	-3.704e-5	15	NC	1	NC	2
94			min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	7720.442	1
95		10	max	.001	1	.004	2	0	15	-3.704e-5	15	NC	1	NC	2
96			min	0	3	-.003	3	-.002	1	-1.115e-3	1	NC	1	9304.473	1
97		11	max	.001	1	.004	2	0	15	-3.704e-5	15	NC	1	NC	1
98			min	0	3	-.003	3	-.002	1	-1.115e-3	1	NC	1	NC	1
99		12	max	.001	1	.003	2	0	15	-3.704e-5	15	NC	1	NC	1
100			min	0	3	-.002	3	-.001	1	-1.115e-3	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	15	-3.704e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-1.115e-3	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-3.704e-5	15	NC	1	NC	1
104			min	0	3	-.002	3	0	1	-1.115e-3	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-3.704e-5	15	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-1.115e-3	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-3.704e-5	15	NC	1	NC	1
108			min	0	3	-.001	3	0	1	-1.115e-3	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-3.704e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-1.115e-3	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.704e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-1.115e-3	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.704e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.115e-3	1	NC	1	NC	1
115	M6	1	max	.011	1	.024	2	.005	1	2.778e-4	1	NC	3	NC	2
116			min	-.009	3	-.018	3	-.002	3	4.557e-6	10	1389.411	2	7269.017	1
117		2	max	.01	1	.022	2	.004	1	2.608e-4	1	NC	3	NC	2
118			min	-.009	3	-.017	3	-.002	3	3.612e-6	10	1483.313	2	7899.215	1
119		3	max	.01	1	.021	2	.004	1	2.438e-4	1	NC	3	NC	2
120			min	-.008	3	-.016	3	-.002	3	2.667e-6	10	1590.488	2	8647.76	1
121		4	max	.009	1	.019	2	.003	1	2.268e-4	1	NC	3	NC	2
122			min	-.008	3	-.015	3	-.002	3	1.721e-6	10	1713.587	2	9545.032	1
123		5	max	.008	1	.018	2	.003	1	2.098e-4	1	NC	3	NC	1
124			min	-.007	3	-.014	3	-.002	3	7.759e-7	10	1856.024	2	NC	1
125		6	max	.008	1	.016	2	.003	1	1.928e-4	1	NC	3	NC	1
126			min	-.007	3	-.013	3	-.001	3	-1.694e-7	10	2022.27	2	NC	1
127		7	max	.007	1	.015	2	.002	1	1.758e-4	1	NC	3	NC	1
128			min	-.006	3	-.012	3	-.001	3	-1.115e-6	10	2218.287	2	NC	1
129		8	max	.007	1	.014	2	.002	1	1.589e-4	1	NC	3	NC	1
130			min	-.006	3	-.011	3	-.001	3	-2.06e-6	10	2452.21	2	NC	1
131		9	max	.006	1	.012	2	.002	1	1.419e-4	1	NC	3	NC	1
132			min	-.005	3	-.01	3	-.001	3	-3.005e-6	10	2735.433	2	NC	1
133		10	max	.005	1	.011	2	.002	1	1.321e-4	3	NC	3	NC	1
134			min	-.005	3	-.009	3	0	3	-3.951e-6	10	3084.41	2	NC	1
135		11	max	.005	1	.009	2	.001	1	1.272e-4	3	NC	3	NC	1
136			min	-.004	3	-.008	3	0	3	-4.896e-6	10	3523.831	2	NC	1
137		12	max	.004	1	.008	2	0	1	1.223e-4	3	NC	3	NC	1
138			min	-.004	3	-.007	3	0	3	-6.481e-6	2	4092.486	2	NC	1
139		13	max	.004	1	.007	2	0	1	1.174e-4	3	NC	3	NC	1
140			min	-.003	3	-.006	3	0	3	-1.17e-5	2	4855.026	2	NC	1
141		14	max	.003	1	.006	2	0	1	1.125e-4	3	NC	3	NC	1
142			min	-.003	3	-.005	3	0	3	-1.693e-5	2	5927.815	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.076e-4	3	NC	3	NC	1
144			min	-.002	3	-.004	3	0	3	-2.215e-5	2	7543.566	2	NC	1
145		16	max	.002	1	.003	2	0	1	1.027e-4	3	NC	1	NC	1
146			min	-.002	3	-.003	3	0	3	-2.737e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	9.785e-5	3	NC	1	NC	1
148			min	-.001	3	-.002	3	0	3	-3.26e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	9.296e-5	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-3.782e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	8.807e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.304e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.949e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-4.03e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.661e-5	2	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-3.021e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.509e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-2.012e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.65e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	2	-1.002e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	0	3	1.791e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	2	5.38e-8	12	8699.754	2	NC	1
163		6	max	0	3	.007	2	0	3	1.932e-5	1	NC	3	NC	1
164			min	0	2	-.008	3	0	2	5.69e-7	15	6968.799	2	NC	1
165		7	max	0	3	.008	2	0	3	2.073e-5	1	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-1.971e-6	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.618e-4	1	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.971e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.618e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.971e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.618e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.007	2	0	3	1.117e-3	1	NC	3	NC	1
230			min	-.003	3	-.006	3	-.002	1	-1.616e-4	3	4579.768	2	NC	1
231		2	max	.003	1	.007	2	0	3	1.058e-3	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.002	1	-1.574e-4	3	4955.596	2	NC	1
233		3	max	.003	1	.006	2	0	3	9.997e-4	1	NC	3	NC	1
234			min	-.002	3	-.006	3	-.002	1	-1.531e-4	3	5395.324	2	NC	1
235		4	max	.003	1	.006	2	0	3	9.41e-4	1	NC	3	NC	1
236			min	-.002	3	-.006	3	-.002	1	-1.489e-4	3	5913.014	2	NC	1
237		5	max	.003	1	.005	2	0	3	8.823e-4	1	NC	3	NC	1
238			min	-.002	3	-.005	3	-.002	1	-1.447e-4	3	6527.045	2	NC	1
239		6	max	.002	1	.005	2	0	3	8.236e-4	1	NC	1	NC	1
240			min	-.002	3	-.005	3	-.002	1	-1.405e-4	3	7261.824	2	NC	1
241		7	max	.002	1	.004	2	0	3	7.649e-4	1	NC	1	NC	1
242			min	-.002	3	-.005	3	-.001	1	-1.363e-4	3	8150.344	2	NC	1
243		8	max	.002	1	.004	2	0	3	7.062e-4	1	NC	1	NC	1
244			min	-.002	3	-.004	3	-.001	1	-1.321e-4	3	9238.179	2	NC	1
245		9	max	.002	1	.003	2	0	3	6.475e-4	1	NC	1	NC	1
246			min	-.002	3	-.004	3	-.001	1	-1.278e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	5.889e-4	1	NC	1	NC	1
248			min	-.001	3	-.004	3	0	1	-1.236e-4	3	NC	1	NC	1
249		11	max	.002	1	.002	2	0	3	5.302e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.194e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.715e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.152e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	4.128e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-1.11e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.541e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.068e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.954e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.025e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.367e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-9.833e-5	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.78e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-9.411e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.193e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-8.989e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.059e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.568e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	3.94e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.948e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	2	2.686e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-1.061e-4	1	NC	1	NC	1
271		3	max	0	9	0	2	0	2	1.432e-5	3	NC	1	NC	1
272			min	0	10	-.002	3	0	3	-1.828e-4	1	NC	1	NC	1
273		4	max	0	9	0	2	0	10	1.784e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	1	-2.595e-4	1	NC	1	NC	1
275		5	max	0	9	0	2	0	10	-7.426e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	-.001	1	-3.361e-4	1	NC	1	NC	1
277		6	max	0	9	0	2	0	15	-1.38e-5	15	NC	1	NC	1
278			min	0	10	-.004	3	-.002	1	-4.128e-4	1	NC	1	NC	1
279		7	max	0	9	.001	2	0	15	-1.641e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-.004	3	-.003	1	-4.895e-4	1	NC	1	NC	1
281		8	max	0	9	.001	2	0	15	-1.902e-5	15	NC	1	NC	1
282			min	0	10	-.005	3	-.004	1	-5.661e-4	1	NC	1	NC	1
283		9	max	0	9	.002	2	0	15	-2.163e-5	15	NC	1	NC	2
284			min	0	10	-.006	3	-.005	1	-6.428e-4	1	NC	1	9038.252	1
285		10	max	0	9	.002	2	0	15	-2.424e-5	15	NC	1	NC	2
286			min	0	10	-.006	3	-.006	1	-7.195e-4	1	NC	1	7288.378	1
287		11	max	0	9	.003	2	0	15	-2.685e-5	15	NC	1	NC	2
288			min	0	10	-.006	3	-.008	1	-7.961e-4	1	NC	1	6058.296	1
289		12	max	0	9	.003	2	0	15	-2.946e-5	15	NC	1	NC	2
290			min	0	10	-.007	3	-.009	1	-8.728e-4	1	NC	1	5161.163	1
291		13	max	0	9	.004	2	0	15	-3.207e-5	15	NC	1	NC	2
292			min	0	10	-.007	3	-.01	1	-9.495e-4	1	NC	1	4487.934	1
293		14	max	0	9	.005	2	0	15	-3.468e-5	15	NC	1	NC	2
294			min	0	10	-.007	3	-.012	1	-1.026e-3	1	9762.413	2	3971.454	1
295		15	max	0	9	.006	2	0	15	-3.729e-5	15	NC	3	NC	2
296			min	0	10	-.007	3	-.013	1	-1.103e-3	1	8270.412	2	3568.548	1
297		16	max	.001	9	.006	2	0	15	-3.99e-5	15	NC	3	NC	3
298			min	0	10	-.007	3	-.014	1	-1.179e-3	1	7099.378	2	3250.486	1
299		17	max	.001	9	.007	2	0	15	-4.251e-5	15	NC	3	NC	3
300			min	0	10	-.007	3	-.015	1	-1.256e-3	1	6172.955	2	2997.628	1
301		18	max	.001	9	.008	2	0	15	-4.512e-5	15	NC	3	NC	3
302			min	0	10	-.007	3	-.016	1	-1.333e-3	1	5434.886	2	2796.282	1
303		19	max	.001	9	.01	2	0	15	-4.773e-5	15	NC	3	NC	3
304			min	0	10	-.007	3	-.017	1	-1.409e-3	1	4843.65	2	2636.799	1
305	M12	1	max	.003	1	.008	2	.014	1	1.236e-3	1	NC	1	NC	3
306			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1343.524	1
307		2	max	.003	1	.008	2	.013	1	1.236e-3	1	NC	1	NC	3
308			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1465.135	1
309		3	max	.002	1	.008	2	.012	1	1.236e-3	1	NC	1	NC	3
310			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1609.897	1
311		4	max	.002	1	.007	2	.011	1	1.236e-3	1	NC	1	NC	3
312			min	0	3	-.005	3	0	15	4.238e-5	15	NC	1	1783.911	1
313		5	max	.002	1	.007	2	.01	1	1.236e-3	1	NC	1	NC	3
314			min	0	3	-.005	3	0	15	4.238e-5	15	NC	1	1995.49	1
315		6	max	.002	1	.006	2	.009	1	1.236e-3	1	NC	1	NC	3
316			min	0	3	-.005	3	0	15	4.238e-5	15	NC	1	2256.194	1
317		7	max	.002	1	.006	2	.007	1	1.236e-3	1	NC	1	NC	3
318			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	2582.493	1
319		8	max	.002	1	.005	2	.006	1	1.236e-3	1	NC	1	NC	3
320			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	2998.5	1
321		9	max	.001	1	.005	2	.005	1	1.236e-3	1	NC	1	NC	3
322			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	3540.696	1
323		10	max	.001	1	.004	2	.005	1	1.236e-3	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	4266.429	1
325		11	max	.001	1	.004	2	.004	1	1.236e-3	1	NC	1	NC	2
326			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	5270.164	1
327		12	max	.001	1	.003	2	.003	1	1.236e-3	1	NC	1	NC	2
328			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	6716.696	1
329		13	max	0	1	.003	2	.002	1	1.236e-3	1	NC	1	NC	2
330			min	0	3	-.002	3	0	15	4.238e-5	15	NC	1	8915.169	1
331		14	max	0	1	.002	2	.002	1	1.236e-3	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	4.238e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	.001	1	1.236e-3	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	4.238e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	1.236e-3	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	4.238e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	1.236e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	4.238e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.236e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	4.238e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.236e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	4.238e-5	15	NC	1	NC	1
343	M1	1	max	.006	3	.022	3	.001	3	1.976e-2	1	NC	1	NC	1
344			min	-.007	2	-.03	1	-.005	1	-1.628e-2	3	NC	1	NC	1
345		2	max	.006	3	.012	3	0	3	9.411e-3	1	NC	4	NC	2
346			min	-.007	2	-.016	1	-.012	1	-8.063e-3	3	3382.628	1	6819.916	1
347		3	max	.006	3	.003	3	0	3	4.029e-6	3	NC	5	NC	2
348			min	-.007	2	-.003	1	-.016	1	-7.451e-4	1	1749.696	1	4133.958	1
349		4	max	.006	3	.008	1	0	3	8.535e-6	3	NC	5	NC	3
350			min	-.007	2	-.005	3	-.019	1	-6.203e-4	1	1238.252	1	3418.895	1
351		5	max	.006	3	.017	1	0	12	1.304e-5	3	NC	5	NC	3
352			min	-.007	2	-.011	3	-.019	1	-4.956e-4	1	992.549	1	3280.297	1
353		6	max	.006	3	.024	1	0	12	1.755e-5	3	NC	5	NC	3
354			min	-.007	2	-.016	3	-.018	1	-3.709e-4	1	853.789	1	3505.663	1
355		7	max	.006	3	.03	1	0	12	2.205e-5	3	NC	5	NC	2
356			min	-.007	2	-.02	3	-.016	1	-2.461e-4	1	769.9	1	4165.641	1
357		8	max	.006	3	.034	1	0	12	2.656e-5	3	NC	5	NC	2
358			min	-.007	2	-.022	3	-.013	1	-1.214e-4	1	719.235	1	5695.422	1
359		9	max	.006	3	.037	1	0	3	3.106e-5	3	NC	5	NC	1
360			min	-.007	2	-.024	3	-.009	1	-8.485e-6	2	691.698	1	NC	1
361		10	max	.006	3	.038	1	0	3	1.281e-4	1	NC	5	NC	1
362			min	-.007	2	-.024	3	-.005	1	4.568e-6	15	682.642	1	NC	1
363		11	max	.006	3	.037	1	0	3	2.528e-4	1	NC	5	NC	1
364			min	-.007	2	-.023	3	-.001	1	8.732e-6	15	690.671	1	NC	1
365		12	max	.006	3	.034	1	.002	1	3.775e-4	1	NC	5	NC	2
366			min	-.007	2	-.021	3	0	15	1.29e-5	15	717.062	1	6711.661	1
367		13	max	.006	3	.03	1	.006	1	5.023e-4	1	NC	5	NC	2
368			min	-.007	2	-.018	3	0	15	1.706e-5	15	766.302	1	4645.728	1
369		14	max	.006	3	.024	1	.008	1	6.27e-4	1	NC	5	NC	3
370			min	-.007	2	-.014	3	0	15	2.122e-5	15	848.202	1	3803.237	1
371		15	max	.006	3	.016	1	.009	1	7.517e-4	1	NC	5	NC	3
372			min	-.007	2	-.01	3	0	15	2.539e-5	15	983.784	1	3499.79	1
373		16	max	.006	3	.007	1	.009	1	8.42e-4	1	NC	5	NC	3
374			min	-.007	2	-.004	3	0	15	2.842e-5	15	1223.152	1	3604.74	1
375		17	max	.006	3	.002	3	.007	1	1.121e-4	1	NC	4	NC	2
376			min	-.007	2	-.005	1	0	15	4.551e-6	15	1716.7	1	4322.88	1
377		18	max	.006	3	.01	3	.003	1	1.103e-2	1	NC	4	NC	2
378			min	-.007	2	-.018	1	0	15	-3.682e-3	3	3308.834	1	7090.646	1
379		19	max	.006	3	.017	3	0	3	2.217e-2	1	NC	1	NC	1
380			min	-.007	2	-.032	1	-.004	1	-7.461e-3	3	NC	1	NC	1
381	M5	1	max	.017	3	.066	3	.001	3	6.379e-7	1	NC	1	NC	1
382			min	-.023	2	-.091	1	-.006	1	4.46e-8	15	NC	1	NC	1
383		2	max	.017	3	.037	3	.002	3	4.404e-5	3	NC	5	NC	1
384			min	-.023	2	-.05	1	-.005	1	-9.111e-5	1	1128.995	1	NC	1
385		3	max	.017	3	.01	3	.002	3	8.674e-5	3	NC	5	NC	1
386			min	-.023	2	-.011	1	-.005	1	-1.809e-4	1	581.284	1	NC	1
387		4	max	.017	3	.022	1	.003	3	8.559e-5	3	NC	5	NC	1
388			min	-.023	2	-.013	3	-.004	1	-1.688e-4	1	410.285	1	NC	1
389		5	max	.017	3	.05	1	.003	3	8.443e-5	3	NC	15	NC	1
390			min	-.024	2	-.031	3	-.003	1	-1.567e-4	1	328.082	1	NC	1
391		6	max	.017	3	.073	1	.003	3	8.327e-5	3	NC	15	NC	1
392			min	-.024	2	-.046	3	-.003	1	-1.446e-4	1	281.575	1	NC	1
393		7	max	.017	3	.091	1	.003	3	8.212e-5	3	NC	15	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.006	3	.002	3	0	15	-1.428e-5	12	NC	4	NC	2
452			min	-.007	2	-.005	1	-.014	1	-6.3e-4	1	1716.733	1	3999.645	1
453		18	max	.006	3	.01	3	0	15	3.686e-3	3	NC	4	NC	2
454			min	-.007	2	-.018	1	-.009	1	-1.13e-2	1	3308.891	1	6663.262	1
455		19	max	.006	3	.017	3	0	3	7.46e-3	3	NC	1	NC	1
456			min	-.007	2	-.032	1	-.002	1	-2.217e-2	1	NC	1	NC	1
457	M13	1	max	.007	1	.022	3	.006	3	3.786e-3	3	NC	1	NC	1
458			min	0	3	-.03	1	-.007	2	-5.231e-3	1	NC	1	NC	1
459		2	max	.007	1	.2	3	.056	1	4.585e-3	3	NC	5	NC	3
460			min	0	3	-.246	1	.001	10	-6.371e-3	1	943.56	1	3361.754	1
461		3	max	.007	1	.346	3	.141	1	5.384e-3	3	NC	5	NC	3
462			min	0	3	-.423	1	.005	15	-7.51e-3	1	519.148	1	1395.413	1
463		4	max	.007	1	.436	3	.214	1	6.183e-3	3	NC	5	NC	3
464			min	0	3	-.534	1	.007	15	-8.649e-3	1	405.036	1	931.609	1
465		5	max	.007	1	.462	3	.25	1	6.981e-3	3	NC	15	NC	3
466			min	0	3	-.566	1	.009	15	-9.789e-3	1	380.757	1	800.121	1
467		6	max	.007	1	.424	3	.239	1	7.78e-3	3	NC	5	NC	3
468			min	0	3	-.521	1	.008	15	-1.093e-2	1	415.658	1	837.546	1
469		7	max	.007	1	.335	3	.183	1	8.579e-3	3	NC	5	NC	3
470			min	0	3	-.414	1	.006	15	-1.207e-2	1	530.94	1	1086.824	1
471		8	max	.007	1	.22	3	.099	1	9.378e-3	3	NC	5	NC	3
472			min	-.001	3	-.276	1	0	10	-1.321e-2	1	830.448	1	1966.136	1
473		9	max	.006	1	.114	3	.017	9	1.018e-2	3	NC	5	NC	2
474			min	-.001	3	-.149	1	-.009	10	-1.435e-2	1	1719.047	1	9439.858	1
475		10	max	.006	1	.066	3	.017	3	1.098e-2	3	NC	4	NC	1
476			min	-.001	3	-.091	1	-.023	2	-1.549e-2	1	3348.318	1	NC	1
477		11	max	.006	1	.114	3	.022	1	1.018e-2	3	NC	5	NC	2
478			min	-.001	3	-.149	1	-.009	10	-1.435e-2	1	1719.048	1	7450.592	1
479		12	max	.006	1	.22	3	.108	1	9.379e-3	3	NC	5	NC	3
480			min	-.001	3	-.276	1	0	10	-1.321e-2	1	830.448	1	1801.977	1
481		13	max	.006	1	.335	3	.194	1	8.58e-3	3	NC	5	NC	5
482			min	-.001	3	-.414	1	.007	15	-1.207e-2	1	530.94	1	1024.99	1
483		14	max	.006	1	.424	3	.25	1	7.782e-3	3	NC	5	NC	5
484			min	-.001	3	-.521	1	.009	15	-1.093e-2	1	415.659	1	799.283	1
485		15	max	.006	1	.462	3	.261	1	6.983e-3	3	NC	15	NC	5
486			min	-.001	3	-.566	1	.009	15	-9.788e-3	1	380.757	1	767.784	1
487		16	max	.006	1	.436	3	.223	1	6.184e-3	3	NC	5	NC	3
488			min	-.001	3	-.534	1	.008	15	-8.648e-3	1	405.036	1	895.48	1
489		17	max	.005	1	.346	3	.148	1	5.386e-3	3	NC	5	NC	3
490			min	-.001	3	-.423	1	.005	15	-7.509e-3	1	519.148	1	1338.409	1
491		18	max	.005	1	.2	3	.059	1	4.587e-3	3	NC	5	NC	3
492			min	-.001	3	-.246	1	.001	10	-6.369e-3	1	943.561	1	3193.888	1
493		19	max	.005	1	.022	3	.006	3	3.789e-3	3	NC	1	NC	1
494			min	-.001	3	-.03	1	-.007	2	-5.229e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.017	3	.006	3	5.437e-3	1	NC	1	NC	1
496			min	0	3	-.032	1	-.007	2	-2.875e-3	3	NC	1	NC	1
497		2	max	.002	1	.099	3	.06	1	6.659e-3	1	NC	5	NC	3
498			min	0	3	-.275	1	.001	10	-3.453e-3	3	840.936	1	3122.657	1
499		3	max	.002	1	.166	3	.149	1	7.882e-3	1	NC	5	NC	3
500			min	0	3	-.473	1	.005	15	-4.032e-3	3	462.708	1	1323.526	1
501		4	max	.002	1	.208	3	.224	1	9.105e-3	1	NC	5	NC	3
502			min	0	3	-.597	1	.008	15	-4.611e-3	3	361.036	1	891.088	1
503		5	max	.003	1	.221	3	.26	1	1.033e-2	1	NC	15	NC	5
504			min	0	3	-.633	1	.009	15	-5.189e-3	3	339.448	1	767.679	1
505		6	max	.003	1	.206	3	.249	1	1.155e-2	1	NC	5	NC	5
506			min	0	3	-.583	1	.009	15	-5.768e-3	3	370.666	1	803.041	1
507		7	max	.003	1	.167	3	.191	1	1.277e-2	1	NC	5	NC	5





Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	1	-.014	15	.014	1	7.994e-3	3	7849.751	15	NC	4
566		min	0	10	-.059	4	-.009	3	-1.166e-2	1	1845.201	4	3106.982	1
567	18	max	0	1	-.007	15	.004	3	8.474e-3	3	NC	15	NC	4
568		min	0	10	-.031	4	-.008	2	-1.239e-2	1	3626.105	4	5530.769	1
569	19	max	.001	1	.004	3	.02	3	8.955e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.025	2	-1.311e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.007	3	3.017e-3	3	NC	1	NC	1
572		min	-.001	1	-.002	1	-.008	2	-3.969e-3	1	NC	1	NC	1
573	2	max	0	10	-.007	15	.007	1	2.883e-3	3	NC	15	NC	2
574		min	-.001	1	-.03	4	0	10	-3.776e-3	1	3626.105	4	8717.228	1
575	3	max	0	10	-.014	15	.016	1	2.749e-3	3	7849.751	15	NC	3
576		min	-.001	1	-.059	4	-.004	3	-3.583e-3	1	1845.201	4	4929.462	1
577	4	max	0	10	-.02	15	.024	1	2.615e-3	3	5385.388	15	NC	4
578		min	0	1	-.085	4	-.007	3	-3.389e-3	1	1265.916	4	3746.852	1
579	5	max	0	10	-.026	15	.028	1	2.481e-3	3	4202.273	15	NC	4
580		min	0	1	-.109	4	-.01	3	-3.196e-3	1	987.807	4	3233.598	1
581	6	max	0	10	-.03	15	.031	1	2.347e-3	3	3536.657	15	NC	4
582		min	0	1	-.13	4	-.011	3	-3.003e-3	1	831.344	4	3008.423	1
583	7	max	0	10	-.034	15	.032	1	2.213e-3	3	3136.376	15	NC	4
584		min	0	1	-.146	4	-.012	3	-2.809e-3	1	737.252	4	2951.743	1
585	8	max	0	10	-.037	15	.032	1	2.079e-3	3	2896.148	15	NC	4
586		min	0	1	-.158	4	-.012	3	-2.616e-3	1	680.783	4	3022.492	1
587	9	max	0	10	-.039	15	.03	1	1.945e-3	3	2766.843	15	NC	4
588		min	0	1	-.165	4	-.011	3	-2.422e-3	1	650.388	4	3214.805	1
589	10	max	0	10	-.039	15	.027	1	1.811e-3	3	2725.937	15	NC	4
590		min	0	1	-.168	4	-.01	3	-2.229e-3	1	640.772	4	3547.882	1
591	11	max	0	10	-.039	15	.024	1	1.677e-3	3	2766.843	15	NC	4
592		min	0	1	-.165	4	-.009	3	-2.036e-3	1	650.388	4	4070.066	1
593	12	max	0	10	-.037	15	.02	1	1.543e-3	3	2896.148	15	NC	3
594		min	0	1	-.158	4	-.007	3	-1.842e-3	1	680.783	4	4877.432	1
595	13	max	0	10	-.034	15	.015	1	1.409e-3	3	3136.376	15	NC	3
596		min	0	1	-.146	4	-.005	3	-1.649e-3	1	737.252	4	6161.487	1
597	14	max	0	10	-.03	15	.011	1	1.275e-3	3	3536.657	15	NC	2
598		min	0	1	-.129	4	-.004	3	-1.456e-3	1	831.344	4	8335.758	1
599	15	max	0	10	-.026	15	.007	1	1.141e-3	3	4202.273	15	NC	1
600		min	0	1	-.109	4	-.002	3	-1.262e-3	1	987.807	4	NC	1
601	16	max	0	10	-.02	15	.004	1	1.007e-3	3	5385.388	15	NC	1
602		min	0	1	-.085	4	-.001	3	-1.069e-3	1	1265.916	4	NC	1
603	17	max	0	10	-.014	15	.001	1	8.728e-4	3	7849.751	15	NC	1
604		min	0	1	-.058	4	0	3	-8.756e-4	1	1845.201	4	NC	1
605	18	max	0	10	-.007	15	0	4	7.388e-4	3	NC	15	NC	1
606		min	0	1	-.03	4	0	2	-6.965e-4	2	3626.105	4	NC	1
607	19	max	0	1	0	1	0	1	6.048e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.383e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

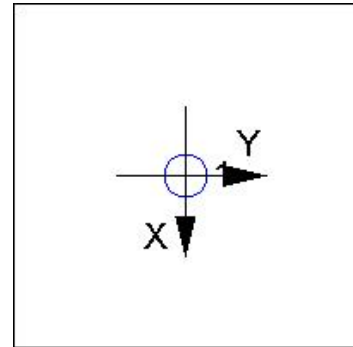
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

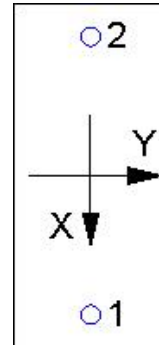
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.