

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.032 k-ft
$P_n$ =	0.166 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>9%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

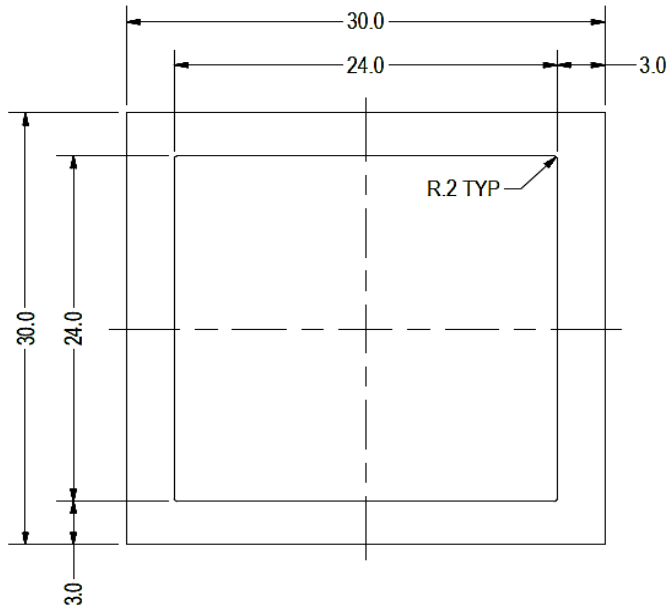
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.624 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

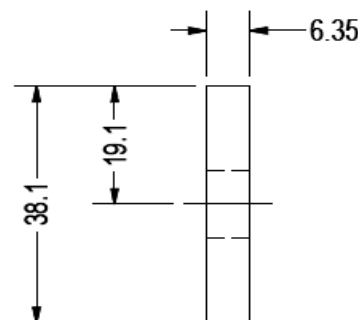
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.700 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>14%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.188 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

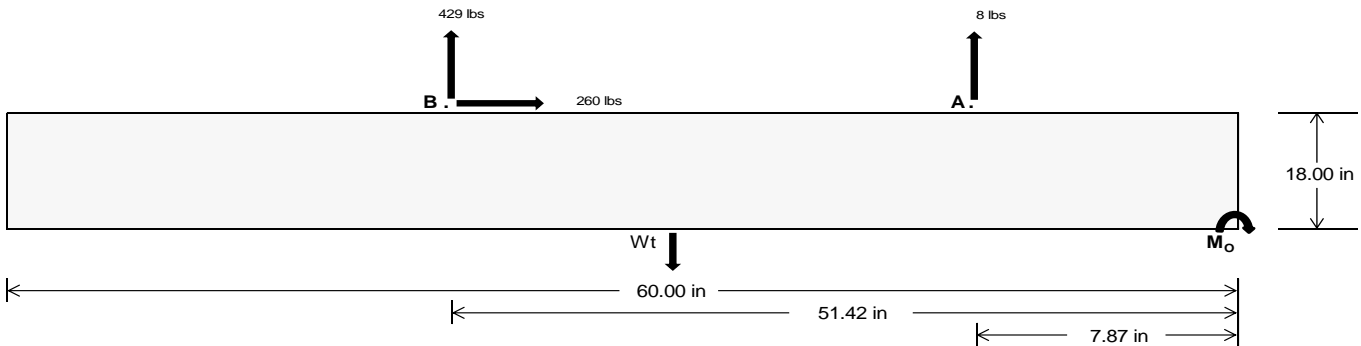
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>38.35</u>	<u>1784.76</u>	k
Compressive Load =	<u>1099.40</u>	<u>1210.43</u>	k
Lateral Load =	<u>25.94</u>	<u>1080.09</u>	k
Moment (Weak Axis) =	<u>0.04</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 26772.3$  in-lbs  
Resisting Force Required = 892.41 lbs  
S.F. = 1.67  
Weight Required = 1487.35 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 259.57 lbs  
Friction = 0.4  
Weight Required = 648.93 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 259.57 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width	22 in	23 in	24 in	25 in
	1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	380 lbs	380 lbs	380 lbs	380 lbs	402 lbs	402 lbs	402 lbs	402 lbs	552 lbs	552 lbs	552 lbs	552 lbs	-17 lbs	-17 lbs	-17 lbs	-17 lbs
$F_B$	260 lbs	260 lbs	260 lbs	260 lbs	528 lbs	528 lbs	528 lbs	528 lbs	567 lbs	567 lbs	567 lbs	567 lbs	-857 lbs	-857 lbs	-857 lbs	-857 lbs
$F_V$	39 lbs	39 lbs	39 lbs	39 lbs	467 lbs	467 lbs	467 lbs	467 lbs	376 lbs	376 lbs	376 lbs	376 lbs	-519 lbs	-519 lbs	-519 lbs	-519 lbs
$P_{total}$	2634 lbs	2725 lbs	2815 lbs	2906 lbs	2924 lbs	3014 lbs	3105 lbs	3195 lbs	3112 lbs	3203 lbs	3293 lbs	3384 lbs	323 lbs	377 lbs	431 lbs	486 lbs
$M$	294 lbs-ft	294 lbs-ft	294 lbs-ft	294 lbs-ft	498 lbs-ft	498 lbs-ft	498 lbs-ft	498 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft
$e$	0.11 ft	0.11 ft	0.10 ft	0.10 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.23 ft	1.91 ft	1.67 ft	1.48 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	248.8 psf	247.4 psf	246.2 psf	245.0 psf	253.7 psf	252.1 psf	250.7 psf	249.3 psf	264.9 psf	262.8 psf	261.0 psf	259.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	325.9 psf	321.2 psf	316.9 psf	312.9 psf	384.2 psf	376.9 psf	370.3 psf	364.2 psf	414.1 psf	405.5 psf	397.7 psf	390.5 psf	439.9 psf	222.6 psf	173.3 psf	152.8 psf

Maximum Bearing Pressure = 440 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

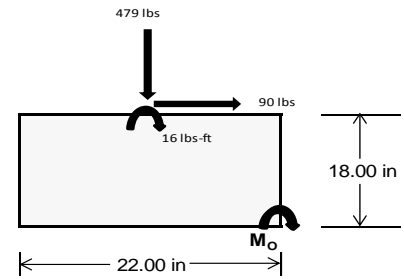
### Overturning Check

$M_o = 287.8 \text{ ft-lbs}$   
 Resisting Force Required = 313.91 lbs  
 S.F. = 1.67  
 Weight Required = 523.19 lbs  
 Minimum Width = 22 in in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	124 lbs	78 lbs	62 lbs	237 lbs	479 lbs	190 lbs	81 lbs	-26 lbs	22 lbs
$F_v$	15 lbs	120 lbs	15 lbs	10 lbs	90 lbs	11 lbs	15 lbs	120 lbs	15 lbs
$P_{total}$	2592 lbs	2546 lbs	2530 lbs	2587 lbs	2829 lbs	2540 lbs	803 lbs	695 lbs	744 lbs
$M$	41 lbs-ft	201 lbs-ft	43 lbs-ft	29 lbs-ft	151 lbs-ft	33 lbs-ft	42 lbs-ft	200 lbs-ft	43 lbs-ft
$e$	0.02 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.29 ft	0.06 ft
$L/6$	0.31 ft	1.68 ft	1.80 ft	1.81 ft	1.73 ft	1.81 ft	1.73 ft	1.26 ft	1.72 ft
$f_{min}$	268.1 sqft	206.1 sqft	260.7 sqft	271.7 sqft	254.5 sqft	265.3 sqft	72.5 sqft	4.3 sqft	65.8 sqft
$f_{max}$	297.3 psf	349.4 psf	291.4 psf	292.7 psf	362.6 psf	288.9 psf	102.6 psf	147.4 psf	96.5 psf



Maximum Bearing Pressure = 363 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

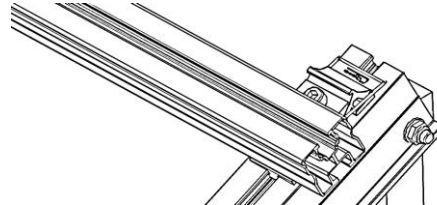
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.657 k
Allowable Uplift =	1.214 k
Utilization =	<u>54%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.112 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.846 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>15%</u>

#### Diagonal Strut

Maximum Axial Load =	0.624 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.146 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.188 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.065 in
	<u>0.065 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$148.425$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$154.13$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.251 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	222.126	1	-.016	15	.201	1	0	10	0	4	0	15
30			min	-365.487	3	-.074	3	-.511	5	0	4	0	3	0	6
31		16	max	222.252	1	-.028	15	.201	1	0	10	0	4	0	15
32			min	-365.392	3	-.114	4	-.626	5	0	4	0	3	0	6
33		17	max	222.378	1	-.04	15	.201	1	0	10	0	4	0	15
34			min	-365.298	3	-.165	4	-.74	5	0	4	0	3	0	6
35		18	max	222.504	1	-.052	15	.201	1	0	10	0	1	0	15
36			min	-365.203	3	-.217	4	-.855	5	0	4	0	3	0	6
37		19	max	222.63	1	-.064	15	.201	1	0	10	0	1	0	15
38			min	-365.109	3	-.268	4	-.969	5	0	4	0	3	0	6
39	M3	1	max	179.651	2	1.756	6	-.011	10	0	5	0	1	0	6
40			min	-176.786	3	.412	15	-1.347	4	0	1	0	10	0	15
41		2	max	179.582	2	1.58	6	-.011	10	0	5	0	1	0	2
42			min	-176.838	3	.371	15	-1.213	4	0	1	0	10	0	12
43		3	max	179.512	2	1.403	6	-.011	10	0	5	0	1	0	2
44			min	-176.89	3	.329	15	-1.079	4	0	1	0	5	0	3
45		4	max	179.443	2	1.226	6	-.011	10	0	5	0	1	0	15
46			min	-176.942	3	.287	15	-.946	4	0	1	0	5	0	4
47		5	max	179.374	2	1.049	6	-.011	10	0	5	0	1	0	15
48			min	-176.994	3	.246	15	-.812	4	0	1	0	5	0	4
49		6	max	179.304	2	.872	6	-.011	10	0	5	0	1	0	15
50			min	-177.046	3	.204	15	-.678	4	0	1	0	5	0	4
51		7	max	179.235	2	.695	6	-.011	10	0	5	0	1	0	15
52			min	-177.098	3	.163	15	-.545	4	0	1	0	5	0	4
53		8	max	179.166	2	.519	6	-.011	10	0	5	0	1	0	15
54			min	-177.15	3	.121	15	-.411	4	0	1	0	5	-.001	4
55		9	max	179.096	2	.342	6	-.011	10	0	5	0	1	0	15
56			min	-177.202	3	.08	15	-.277	4	0	1	0	5	-.001	4
57		10	max	179.027	2	.165	6	-.011	10	0	5	0	1	0	15
58			min	-177.254	3	.038	15	-.225	1	0	1	0	5	-.001	4
59		11	max	178.958	2	.018	2	.041	5	0	5	0	1	0	15
60			min	-177.306	3	-.038	3	-.225	1	0	1	0	5	-.001	4
61		12	max	178.888	2	-.045	15	.175	5	0	5	0	1	0	15
62			min	-177.358	3	-.189	4	-.225	1	0	1	0	5	-.001	4
63		13	max	178.819	2	-.087	15	.309	5	0	5	0	1	0	15
64			min	-177.41	3	-.366	4	-.225	1	0	1	0	5	-.001	4
65		14	max	178.75	2	-.128	15	.442	5	0	5	0	1	0	15
66			min	-177.462	3	-.543	4	-.225	1	0	1	0	5	-.001	4
67		15	max	178.68	2	-.17	15	.576	5	0	5	0	1	0	15
68			min	-177.514	3	-.719	4	-.225	1	0	1	0	5	0	4
69		16	max	178.611	2	-.211	15	.71	5	0	5	0	9	0	15
70			min	-177.566	3	-.896	4	-.225	1	0	1	0	5	0	4
71		17	max	178.542	2	-.253	15	.843	5	0	5	0	10	0	15
72			min	-177.618	3	-1.073	4	-.225	1	0	1	0	4	0	4
73		18	max	178.473	2	-.295	15	.977	5	0	5	0	10	0	15
74			min	-177.67	3	-1.25	4	-.225	1	0	1	0	4	0	4
75		19	max	178.403	2	-.336	15	1.111	5	0	5	0	5	0	1
76			min	-177.722	3	-1.427	4	-.225	1	0	1	0	1	0	1
77	M4	1	max	309.76	1	0	1	-.04	10	0	1	0	5	0	1
78			min	2.408	12	0	1	-18.853	4	0	1	0	2	0	1
79		2	max	309.825	1	0	1	-.04	10	0	1	0	12	0	1
80			min	2.44	12	0	1	-18.91	4	0	1	-.002	4	0	1
81		3	max	309.89	1	0	1	-.04	10	0	1	0	10	0	1
82			min	2.473	12	0	1	-18.966	4	0	1	-.003	4	0	1
83		4	max	309.954	1	0	1	-.04	10	0	1	0	10	0	1
84			min	2.505	12	0	1	-19.022	4	0	1	-.005	4	0	1
85		5	max	310.019	1	0	1	-.04	10	0	1	0	10	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	2.537	12	0	1	-19.078	4	0	1	-.007	4	0	1
87		6	max	310.084	1	0	1	-.04	10	0	1	0	10	0	1
88			min	2.57	12	0	1	-19.134	4	0	1	-.008	4	0	1
89		7	max	310.148	1	0	1	-.04	10	0	1	0	10	0	1
90			min	2.602	12	0	1	-19.19	4	0	1	-.01	4	0	1
91		8	max	310.213	1	0	1	-.04	10	0	1	0	10	0	1
92			min	2.635	12	0	1	-19.246	4	0	1	-.012	4	0	1
93		9	max	310.278	1	0	1	-.04	10	0	1	0	10	0	1
94			min	2.667	12	0	1	-19.302	4	0	1	-.014	4	0	1
95		10	max	310.343	1	0	1	-.04	10	0	1	0	10	0	1
96			min	2.699	12	0	1	-19.358	4	0	1	-.015	4	0	1
97		11	max	310.407	1	0	1	-.04	10	0	1	0	10	0	1
98			min	2.732	12	0	1	-19.414	4	0	1	-.017	4	0	1
99		12	max	310.472	1	0	1	-.04	10	0	1	0	10	0	1
100			min	2.764	12	0	1	-19.47	4	0	1	-.019	4	0	1
101		13	max	310.537	1	0	1	-.04	10	0	1	0	10	0	1
102			min	2.796	12	0	1	-19.526	4	0	1	-.021	4	0	1
103		14	max	310.601	1	0	1	-.04	10	0	1	0	10	0	1
104			min	2.829	12	0	1	-19.582	4	0	1	-.022	4	0	1
105		15	max	310.666	1	0	1	-.04	10	0	1	0	10	0	1
106			min	2.861	12	0	1	-19.639	4	0	1	-.024	4	0	1
107		16	max	310.731	1	0	1	-.04	10	0	1	0	10	0	1
108			min	2.893	12	0	1	-19.695	4	0	1	-.026	4	0	1
109		17	max	310.796	1	0	1	-.04	10	0	1	0	10	0	1
110			min	2.926	12	0	1	-19.751	4	0	1	-.028	4	0	1
111		18	max	310.86	1	0	1	-.04	10	0	1	0	10	0	1
112			min	2.958	12	0	1	-19.807	4	0	1	-.029	4	0	1
113		19	max	310.925	1	0	1	-.04	10	0	1	0	10	0	1
114			min	2.99	12	0	1	-19.863	4	0	1	-.031	4	0	1
115	M6	1	max	698.172	1	.64	6	1.077	4	0	3	0	3	0	1
116			min	-1146.124	3	.143	15	-.234	3	0	5	0	2	0	1
117		2	max	698.298	1	.589	6	.962	4	0	3	0	4	0	15
118			min	-1146.029	3	.131	15	-.234	3	0	5	0	2	0	6
119		3	max	698.424	1	.538	6	.848	4	0	3	0	4	0	15
120			min	-1145.935	3	.119	15	-.234	3	0	5	0	2	0	6
121		4	max	698.55	1	.491	2	.733	4	0	3	0	4	0	15
122			min	-1145.84	3	.107	15	-.234	3	0	5	0	2	0	6
123		5	max	698.676	1	.451	2	.619	4	0	3	0	4	0	15
124			min	-1145.746	3	.095	15	-.234	3	0	5	0	2	0	6
125		6	max	698.802	1	.411	2	.504	4	0	3	0	4	0	15
126			min	-1145.652	3	.082	12	-.234	3	0	5	0	2	0	6
127		7	max	698.928	1	.371	2	.39	4	0	3	0	4	0	15
128			min	-1145.557	3	.062	12	-.234	3	0	5	0	3	0	2
129		8	max	699.054	1	.332	2	.276	4	0	3	0	4	0	15
130			min	-1145.463	3	.042	12	-.234	3	0	5	0	3	0	2
131		9	max	699.179	1	.292	2	.161	4	0	3	0	4	0	15
132			min	-1145.368	3	.023	12	-.234	3	0	5	0	3	0	2
133		10	max	699.305	1	.252	2	.053	14	0	3	0	4	0	15
134			min	-1145.274	3	-.003	3	-.234	3	0	5	0	3	0	2
135		11	max	699.431	1	.212	2	.045	9	0	3	0	4	0	12
136			min	-1145.18	3	-.033	3	-.234	3	0	5	0	3	0	2
137		12	max	699.557	1	.172	2	.045	9	0	3	0	4	0	12
138			min	-1145.085	3	-.063	3	-.234	3	0	5	0	3	0	2
139		13	max	699.683	1	.132	2	.045	9	0	3	0	4	0	12
140			min	-1144.991	3	-.093	3	-.314	5	0	5	0	3	0	2
141		14	max	699.809	1	.092	2	.045	9	0	3	0	4	0	12
142			min	-1144.896	3	-.123	3	-.429	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	699.935	1	.053	2	.045	9	0	3	0	4	0	12
144			min	-1144.802	3	-.153	3	-.543	5	0	5	0	3	0	2
145		16	max	700.06	1	.013	2	.045	9	0	3	0	4	0	12
146			min	-1144.708	3	-.183	3	-.657	5	0	5	0	3	0	2
147		17	max	700.186	1	-.027	2	.045	9	0	3	0	4	0	12
148			min	-1144.613	3	-.213	3	-.772	5	0	5	0	3	0	2
149		18	max	700.312	1	-.062	15	.045	9	0	3	0	4	0	3
150			min	-1144.519	3	-.243	3	-.886	5	0	5	0	3	0	2
151		19	max	700.438	1	-.074	15	.045	9	0	3	0	14	0	3
152			min	-1144.424	3	-.281	4	-1.001	5	0	5	0	3	0	2
153	M7	1	max	624.123	2	1.775	4	.031	3	0	1	0	4	0	2
154			min	-524.288	3	.423	15	-1.328	4	0	3	0	3	0	3
155		2	max	624.054	2	1.598	4	.031	3	0	1	0	4	0	2
156			min	-524.34	3	.382	15	-1.194	4	0	3	0	3	0	3
157		3	max	623.984	2	1.421	4	.031	3	0	1	0	1	0	2
158			min	-524.392	3	.34	15	-1.061	4	0	3	0	3	0	3
159		4	max	623.915	2	1.244	4	.031	3	0	1	0	1	0	2
160			min	-524.444	3	.298	15	-.927	4	0	3	0	3	0	3
161		5	max	623.846	2	1.067	4	.031	3	0	1	0	1	0	15
162			min	-524.496	3	.257	15	-.793	4	0	3	0	5	0	3
163		6	max	623.776	2	.89	4	.031	3	0	1	0	1	0	15
164			min	-524.548	3	.215	15	-.66	4	0	3	0	5	0	3
165		7	max	623.707	2	.714	4	.031	3	0	1	0	1	0	15
166			min	-524.6	3	.174	15	-.526	4	0	3	0	5	0	6
167		8	max	623.638	2	.537	4	.031	3	0	1	0	1	0	15
168			min	-524.652	3	.132	15	-.392	4	0	3	0	5	-.001	6
169		9	max	623.568	2	.36	4	.031	3	0	1	0	1	0	15
170			min	-524.704	3	.071	12	-.259	4	0	3	0	5	-.001	6
171		10	max	623.499	2	.216	2	.031	3	0	1	0	1	0	15
172			min	-524.756	3	-.003	3	-.125	4	0	3	0	5	-.001	6
173		11	max	623.43	2	.078	2	.031	3	0	1	0	1	0	15
174			min	-524.808	3	-.106	3	-.009	1	0	3	0	5	-.001	6
175		12	max	623.36	2	-.034	15	.143	5	0	1	0	1	0	15
176			min	-524.86	3	-.21	3	-.009	1	0	3	0	5	-.001	6
177		13	max	623.291	2	-.076	15	.277	5	0	1	0	1	0	15
178			min	-524.912	3	-.348	6	-.009	1	0	3	0	5	-.001	6
179		14	max	623.222	2	-.117	15	.41	5	0	1	0	1	0	15
180			min	-524.964	3	-.525	6	-.009	1	0	3	0	5	-.001	6
181		15	max	623.152	2	-.159	15	.544	5	0	1	0	1	0	15
182			min	-525.015	3	-.702	6	-.009	1	0	3	0	5	0	6
183		16	max	623.083	2	-.2	15	.678	5	0	1	0	1	0	15
184			min	-525.067	3	-.878	6	-.009	1	0	3	0	5	0	6
185		17	max	623.014	2	-.242	15	.811	5	0	1	0	1	0	15
186			min	-525.119	3	-1.055	6	-.009	1	0	3	0	5	0	6
187		18	max	622.944	2	-.283	15	.945	5	0	1	0	1	0	15
188			min	-525.171	3	-1.232	6	-.009	1	0	3	0	3	0	6
189		19	max	622.875	2	-.325	15	1.079	5	0	1	0	1	0	1
190			min	-525.223	3	-1.409	6	-.009	1	0	3	0	3	0	1
191	M8	1	max	844.528	1	0	1	.323	1	0	1	0	4	0	1
192			min	-30.374	3	0	1	-19.103	4	0	1	0	3	0	1
193		2	max	844.592	1	0	1	.323	1	0	1	0	1	0	1
194			min	-30.326	3	0	1	-19.159	4	0	1	-.002	4	0	1
195		3	max	844.657	1	0	1	.323	1	0	1	0	1	0	1
196			min	-30.277	3	0	1	-19.215	4	0	1	-.003	4	0	1
197		4	max	844.722	1	0	1	.323	1	0	1	0	1	0	1
198			min	-30.229	3	0	1	-19.271	4	0	1	-.005	4	0	1
199		5	max	844.786	1	0	1	.323	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-30.18	3	0	1	-19.327	4	0	1	-.007	4	0	1
201		6	max	844.851	1	0	1	.323	1	0	1	0	1	0	1
202			min	-30.132	3	0	1	-19.384	4	0	1	-.009	4	0	1
203		7	max	844.916	1	0	1	.323	1	0	1	0	1	0	1
204			min	-30.083	3	0	1	-19.44	4	0	1	-.01	4	0	1
205		8	max	844.981	1	0	1	.323	1	0	1	0	1	0	1
206			min	-30.034	3	0	1	-19.496	4	0	1	-.012	4	0	1
207		9	max	845.045	1	0	1	.323	1	0	1	0	1	0	1
208			min	-29.986	3	0	1	-19.552	4	0	1	-.014	4	0	1
209		10	max	845.11	1	0	1	.323	1	0	1	0	1	0	1
210			min	-29.937	3	0	1	-19.608	4	0	1	-.016	4	0	1
211		11	max	845.175	1	0	1	.323	1	0	1	0	1	0	1
212			min	-29.889	3	0	1	-19.664	4	0	1	-.017	4	0	1
213		12	max	845.239	1	0	1	.323	1	0	1	0	1	0	1
214			min	-29.84	3	0	1	-19.72	4	0	1	-.019	4	0	1
215		13	max	845.304	1	0	1	.323	1	0	1	0	1	0	1
216			min	-29.792	3	0	1	-19.776	4	0	1	-.021	4	0	1
217		14	max	845.369	1	0	1	.323	1	0	1	0	1	0	1
218			min	-29.743	3	0	1	-19.832	4	0	1	-.023	4	0	1
219		15	max	845.433	1	0	1	.323	1	0	1	0	1	0	1
220			min	-29.695	3	0	1	-19.888	4	0	1	-.024	4	0	1
221		16	max	845.498	1	0	1	.323	1	0	1	0	1	0	1
222			min	-29.646	3	0	1	-19.944	4	0	1	-.026	4	0	1
223		17	max	845.563	1	0	1	.323	1	0	1	0	1	0	1
224			min	-29.598	3	0	1	-20	4	0	1	-.028	4	0	1
225		18	max	845.628	1	0	1	.323	1	0	1	0	1	0	1
226			min	-29.549	3	0	1	-20.057	4	0	1	-.03	4	0	1
227		19	max	845.692	1	0	1	.323	1	0	1	0	1	0	1
228			min	-29.501	3	0	1	-20.113	4	0	1	-.032	4	0	1
229	M10	1	max	222.403	1	.684	4	1.204	5	0	1	0	1	0	1
230			min	-314.414	3	.174	15	-.12	1	-.001	5	0	3	0	1
231		2	max	222.529	1	.633	4	1.09	5	0	1	0	1	0	15
232			min	-314.319	3	.162	15	-.12	1	-.001	5	0	3	0	4
233		3	max	222.655	1	.582	4	.975	5	0	1	0	4	0	15
234			min	-314.225	3	.15	15	-.12	1	-.001	5	0	3	0	4
235		4	max	222.781	1	.531	4	.861	5	0	1	0	4	0	15
236			min	-314.13	3	.138	15	-.12	1	-.001	5	0	3	0	4
237		5	max	222.907	1	.479	4	.746	5	0	1	0	4	0	15
238			min	-314.036	3	.126	15	-.12	1	-.001	5	0	3	0	4
239		6	max	223.033	1	.428	4	.632	5	0	1	0	4	0	15
240			min	-313.942	3	.114	15	-.12	1	-.001	5	0	3	0	4
241		7	max	223.159	1	.377	4	.518	5	0	1	0	4	0	15
242			min	-313.847	3	.101	15	-.12	1	-.001	5	0	3	0	4
243		8	max	223.284	1	.326	4	.403	5	0	1	0	4	0	15
244			min	-313.753	3	.089	15	-.12	1	-.001	5	0	3	0	4
245		9	max	223.41	1	.275	4	.289	5	0	1	0	4	0	15
246			min	-313.658	3	.077	15	-.12	1	-.001	5	0	3	0	4
247		10	max	223.536	1	.224	4	.174	5	0	1	.001	4	0	15
248			min	-313.564	3	.065	15	-.12	1	-.001	5	0	3	0	4
249		11	max	223.662	1	.173	4	.06	5	0	1	.001	4	0	15
250			min	-313.47	3	.046	12	-.12	1	-.001	5	0	3	0	4
251		12	max	223.788	1	.122	4	-.007	12	0	1	.001	5	0	15
252			min	-313.375	3	.026	12	-.12	1	-.001	5	0	3	0	4
253		13	max	223.914	1	.07	4	-.007	12	0	1	.001	5	0	15
254			min	-313.281	3	.006	12	-.187	4	-.001	5	0	3	0	4
255		14	max	224.04	1	.026	5	-.007	12	0	1	0	5	0	15
256			min	-313.186	3	-.022	3	-.301	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	224.165	1	.007	5	-.007	12	0	1	0	5	0	15
258		min	-313.092	3	-.052	3	-.415	4	-.001	5	0	3	0	4
259	16	max	224.291	1	-.007	15	-.007	12	0	1	0	5	0	15
260		min	-312.998	3	-.084	6	-.53	4	-.001	5	0	3	0	4
261	17	max	224.417	1	-.019	15	-.007	12	0	1	0	5	0	15
262		min	-312.903	3	-.135	6	-.644	4	-.001	5	0	3	0	4
263	18	max	224.543	1	-.031	15	-.007	12	0	1	0	5	0	15
264		min	-312.809	3	-.186	6	-.759	4	-.001	5	0	3	0	4
265	19	max	224.669	1	-.043	15	-.007	12	0	1	0	5	0	15
266		min	-312.714	3	-.238	6	-.873	4	-.001	5	0	1	0	4
267	M11	1	max	179.174	2	1.747	.242	1	0	4	0	5	0	2
268		min	-177.506	3	.405	15	-1.251	5	0	10	0	1	0	15
269	2	max	179.104	2	1.57	6	.242	1	0	4	0	5	0	2
270		min	-177.558	3	.364	15	-1.117	5	0	10	0	1	0	3
271	3	max	179.035	2	1.393	6	.242	1	0	4	0	3	0	2
272		min	-177.61	3	.322	15	-.984	5	0	10	0	1	0	3
273	4	max	178.966	2	1.216	6	.242	1	0	4	0	3	0	15
274		min	-177.662	3	.281	15	-.85	5	0	10	0	1	0	4
275	5	max	178.897	2	1.039	6	.242	1	0	4	0	3	0	15
276		min	-177.714	3	.239	15	-.716	5	0	10	0	1	0	4
277	6	max	178.827	2	.863	6	.242	1	0	4	0	3	0	15
278		min	-177.766	3	.198	15	-.583	5	0	10	0	1	0	4
279	7	max	178.758	2	.686	6	.242	1	0	4	0	3	0	15
280		min	-177.818	3	.156	15	-.449	5	0	10	0	1	0	4
281	8	max	178.689	2	.509	6	.242	1	0	4	0	3	0	15
282		min	-177.87	3	.114	15	-.315	5	0	10	0	4	-.001	4
283	9	max	178.619	2	.332	6	.242	1	0	4	0	3	0	15
284		min	-177.922	3	.073	15	-.182	5	0	10	0	4	-.001	4
285	10	max	178.55	2	.156	2	.242	1	0	4	0	3	0	15
286		min	-177.974	3	.031	15	-.048	5	0	10	0	4	-.001	4
287	11	max	178.481	2	.018	2	.242	1	0	4	0	3	0	15
288		min	-178.026	3	-.051	3	-.044	3	0	10	0	4	-.001	4
289	12	max	178.411	2	-.052	15	.275	4	0	4	0	3	0	15
290		min	-178.078	3	-.199	4	-.044	3	0	10	0	4	-.001	4
291	13	max	178.342	2	-.093	15	.409	4	0	4	0	3	0	15
292		min	-178.13	3	-.376	4	-.044	3	0	10	0	4	-.001	4
293	14	max	178.273	2	-.135	15	.542	4	0	4	0	3	0	15
294		min	-178.182	3	-.553	4	-.044	3	0	10	0	4	-.001	4
295	15	max	178.203	2	-.177	15	.676	4	0	4	0	3	0	15
296		min	-178.234	3	-.729	4	-.044	3	0	10	0	5	0	4
297	16	max	178.134	2	-.218	15	.81	4	0	4	0	3	0	15
298		min	-178.286	3	-.906	4	-.044	3	0	10	0	5	0	4
299	17	max	178.065	2	-.26	15	.943	4	0	4	0	3	0	15
300		min	-178.338	3	-1.083	4	-.044	3	0	10	0	10	0	4
301	18	max	177.995	2	-.301	15	1.077	4	0	4	0	4	0	15
302		min	-178.39	3	-1.26	4	-.044	3	0	10	0	10	0	4
303	19	max	177.926	2	-.343	15	1.211	4	0	4	0	4	0	1
304		min	-178.442	3	-1.437	4	-.044	3	0	10	0	10	0	1
305	M12	1	max	309.74	1	0	1.387	1	0	1	0	4	0	1
306		min	2.516	15	0	1	-17.525	5	0	1	0	3	0	1
307	2	max	309.804	1	0	1	1.387	1	0	1	0	1	0	1
308		min	2.535	15	0	1	-17.581	5	0	1	-.002	5	0	1
309	3	max	309.869	1	0	1	1.387	1	0	1	0	1	0	1
310		min	2.555	15	0	1	-17.638	5	0	1	-.003	5	0	1
311	4	max	309.934	1	0	1	1.387	1	0	1	0	1	0	1
312		min	2.574	15	0	1	-17.694	5	0	1	-.005	5	0	1
313	5	max	309.999	1	0	1	1.387	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	2.594	15	0	1	-17.75	5	0	1	-.006	5	0	1
315		6	max	310.063	1	0	1	1.387	1	0	1	0	1	0	1
316			min	2.613	15	0	1	-17.806	5	0	1	-.008	5	0	1
317		7	max	310.128	1	0	1	1.387	1	0	1	0	1	0	1
318			min	2.633	15	0	1	-17.862	5	0	1	-.009	5	0	1
319		8	max	310.193	1	0	1	1.387	1	0	1	0	1	0	1
320			min	2.652	15	0	1	-17.918	5	0	1	-.011	5	0	1
321		9	max	310.257	1	0	1	1.387	1	0	1	.001	1	0	1
322			min	2.672	15	0	1	-17.974	5	0	1	-.013	5	0	1
323		10	max	310.322	1	0	1	1.387	1	0	1	.001	1	0	1
324			min	2.691	15	0	1	-18.03	5	0	1	-.014	5	0	1
325		11	max	310.387	1	0	1	1.387	1	0	1	.001	1	0	1
326			min	2.711	15	0	1	-18.086	5	0	1	-.016	5	0	1
327		12	max	310.452	1	0	1	1.387	1	0	1	.001	1	0	1
328			min	2.73	15	0	1	-18.142	5	0	1	-.018	5	0	1
329		13	max	310.516	1	0	1	1.387	1	0	1	.002	1	0	1
330			min	2.75	15	0	1	-18.198	5	0	1	-.019	5	0	1
331		14	max	310.581	1	0	1	1.387	1	0	1	.002	1	0	1
332			min	2.769	15	0	1	-18.254	5	0	1	-.021	5	0	1
333		15	max	310.646	1	0	1	1.387	1	0	1	.002	1	0	1
334			min	2.789	15	0	1	-18.31	5	0	1	-.022	5	0	1
335		16	max	310.71	1	0	1	1.387	1	0	1	.002	1	0	1
336			min	2.808	15	0	1	-18.367	5	0	1	-.024	5	0	1
337		17	max	310.775	1	0	1	1.387	1	0	1	.002	1	0	1
338			min	2.828	15	0	1	-18.423	5	0	1	-.026	5	0	1
339		18	max	310.84	1	0	1	1.387	1	0	1	.002	1	0	1
340			min	2.847	15	0	1	-18.479	5	0	1	-.027	5	0	1
341		19	max	310.904	1	0	1	1.387	1	0	1	.002	1	0	1
342			min	2.867	15	0	1	-18.535	5	0	1	-.029	5	0	1
343	M1	1	max	92.896	1	345.269	3	-1.336	10	0	2	.057	1	0	2
344			min	6.495	12	-231.579	2	-28.807	1	0	3	.003	10	0	3
345		2	max	93.036	1	345.088	3	-1.336	10	0	2	.05	1	.051	2
346			min	6.565	12	-231.821	2	-28.807	1	0	3	.002	10	-.075	3
347		3	max	89.446	3	5.132	14	-1.328	10	0	12	.044	1	.1	2
348			min	-15.015	10	-24.676	2	-28.71	1	0	1	.002	10	-.149	3
349		4	max	89.55	3	4.894	14	-1.328	10	0	12	.037	1	.105	2
350			min	-14.899	10	-24.918	2	-28.71	1	0	1	.002	10	-.146	3
351		5	max	89.655	3	4.657	14	-1.328	10	0	12	.031	1	.111	2
352			min	-14.782	10	-25.16	2	-28.71	1	0	1	.001	10	-.143	3
353		6	max	89.76	3	4.419	14	-1.328	10	0	12	.025	1	.116	2
354			min	-14.666	10	-25.402	2	-28.71	1	0	1	.001	10	-.14	3
355		7	max	89.865	3	4.182	14	-1.328	10	0	12	.019	1	.122	2
356			min	-14.55	10	-25.644	2	-28.71	1	0	1	0	10	-.137	3
357		8	max	89.969	3	3.944	14	-1.328	10	0	12	.013	1	.127	2
358			min	-14.433	10	-25.885	2	-28.71	1	0	1	0	10	-.134	3
359		9	max	90.074	3	3.706	14	-1.328	10	0	12	.006	1	.133	2
360			min	-14.317	10	-26.127	2	-28.71	1	0	1	0	10	-.131	3
361		10	max	90.179	3	3.504	9	-1.328	10	0	12	.002	3	.139	2
362			min	-14.201	10	-26.369	2	-28.71	1	0	1	0	10	-.127	3
363		11	max	90.283	3	3.302	9	-1.328	10	0	12	0	3	.144	2
364			min	-14.084	10	-26.611	2	-28.71	1	0	1	-.006	1	-.124	3
365		12	max	90.388	3	3.1	9	-1.328	10	0	12	0	12	.15	2
366			min	-13.968	10	-26.853	2	-28.71	1	0	1	-.012	1	-.121	3
367		13	max	90.493	3	2.899	9	-1.328	10	0	12	0	10	.156	2
368			min	-13.852	10	-27.094	2	-28.71	1	0	1	-.019	1	-.118	3
369		14	max	90.598	3	2.697	9	-1.328	10	0	12	-.001	10	.162	2
370			min	-13.735	10	-27.336	2	-28.71	1	0	1	-.025	1	-.115	3







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-14.387	10	-25.13	2	-22.625	5	0	5	-.031	1	-.143	3
429	6	max	89.483	3	4.291	9	28.15	1	0	1	.038	5	-.116	2
430		min	-14.27	10	-25.372	2	-22.383	5	0	5	-.024	1	-.14	3
431	7	max	89.588	3	4.09	9	28.15	1	0	1	.033	5	.122	2
432		min	-14.154	10	-25.614	2	-22.141	5	0	5	-.018	1	-.137	3
433	8	max	89.692	3	3.888	9	28.15	1	0	1	.029	5	.127	2
434		min	-14.038	10	-25.855	2	-21.899	5	0	5	-.012	1	-.134	3
435	9	max	89.797	3	3.687	9	28.15	1	0	1	.024	5	.133	2
436		min	-13.921	10	-26.097	2	-21.657	5	0	5	-.006	1	-.131	3
437	10	max	89.902	3	3.485	9	28.15	1	0	1	.019	4	.139	2
438		min	-13.805	10	-26.339	2	-21.415	5	0	5	0	1	-.127	3
439	11	max	90.007	3	3.284	9	28.15	1	0	1	.016	4	.144	2
440		min	-13.689	10	-26.581	2	-21.173	5	0	5	0	10	-.124	3
441	12	max	90.111	3	3.082	9	28.15	1	0	1	.013	4	.15	2
442		min	-13.572	10	-26.823	2	-20.931	5	0	5	0	10	-.121	3
443	13	max	90.216	3	2.881	9	28.15	1	0	1	.018	1	.156	2
444		min	-13.456	10	-27.064	2	-20.689	5	0	5	0	10	-.118	3
445	14	max	90.321	3	2.679	9	28.15	1	0	1	.024	1	.162	2
446		min	-13.339	10	-27.306	2	-20.447	5	0	5	0	15	-.115	3
447	15	max	90.425	3	2.477	9	28.15	1	0	1	.03	1	.168	2
448		min	-13.223	10	-27.548	2	-20.205	5	0	5	-.003	5	-.111	3
449	16	max	91.1	2	120.676	2	28.361	1	0	10	.037	1	.173	2
450		min	-6.717	3	-165.143	3	-18.808	5	0	4	-.007	5	-.107	3
451	17	max	91.24	2	120.434	2	28.361	1	0	10	.043	1	.146	2
452		min	-6.612	3	-165.325	3	-18.566	5	0	4	-.011	5	-.071	3
453	18	max	4.989	5	338.912	2	29.761	1	0	2	.05	1	.074	2
454		min	-92.773	1	-162.656	3	-35.832	5	0	3	-.018	5	-.036	3
455	19	max	5.054	5	338.67	2	29.761	1	0	2	.056	1	0	2
456		min	-92.633	1	-162.837	3	-35.59	5	0	3	-.026	5	0	3
457	M13	1	max	134.996	4	231.468	2	-1.796	15	0	.056	1	0	2
458		min	1.337	10	-345.235	3	-92.635	1	0	3	0	15	0	3
459	2	max	129.813	4	164.326	2	-.894	15	0	2	.013	1	.156	3
460		min	1.337	10	-244.609	3	-70.222	1	0	3	-.001	10	-.104	2
461	3	max	124.629	4	97.184	2	.008	15	0	2	.008	3	.258	3
462		min	1.337	10	-143.984	3	-47.81	1	0	3	-.018	1	-.173	2
463	4	max	119.445	4	30.041	2	1.292	5	0	2	.005	3	.308	3
464		min	1.337	10	-43.358	3	-25.398	1	0	3	-.037	1	-.207	2
465	5	max	114.262	4	57.268	3	2.687	5	0	2	.003	3	.304	3
466		min	1.337	10	-37.101	2	-4.125	3	0	3	-.045	1	-.205	2
467	6	max	109.078	4	157.894	3	19.427	1	0	2	.003	5	.247	3
468		min	1.337	10	-104.243	2	-2.812	3	0	3	-.041	1	-.168	2
469	7	max	103.895	4	258.52	3	41.839	1	0	2	.005	5	.137	3
470		min	1.337	10	-171.386	2	-1.499	3	0	3	-.024	1	-.095	2
471	8	max	98.711	4	359.146	3	64.252	1	0	2	.009	4	.014	1
472		min	1.337	10	-238.528	2	-.186	3	0	3	0	3	-.026	3
473	9	max	93.527	4	459.772	3	86.664	1	0	2	.043	1	.157	2
474		min	1.337	10	-305.67	2	.912	12	0	3	0	3	-.242	3
475	10	max	88.344	4	560.398	3	109.076	1	0	2	.095	1	.336	2
476		min	1.337	10	-372.813	2	1.788	12	0	3	-.008	3	-.511	3
477	11	max	62.199	4	305.67	2	3.497	5	0	3	.043	1	.157	2
478		min	1.337	10	-459.772	3	-86.409	1	0	2	-.013	5	-.242	3
479	12	max	57.015	4	238.528	2	4.893	5	0	3	.005	2	.014	1
480		min	1.337	10	-359.146	3	-63.997	1	0	2	-.011	5	-.026	3
481	13	max	51.831	4	171.386	2	6.289	5	0	3	-.001	10	.137	3
482		min	1.337	10	-258.52	3	-41.585	1	0	2	-.025	1	-.095	2
483	14	max	46.648	4	104.243	2	7.684	5	0	3	-.003	15	.247	3
484		min	1.337	10	-157.894	3	-19.173	1	0	2	-.041	1	-.168	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	41.464	4	37.101	2	10.182	4	0	3	0	15	.304	3
486			min	1.337	10	-57.268	3	-1.308	10	0	2	-.045	1	-.205	2
487		16	max	36.28	4	43.358	3	25.652	1	0	3	.005	5	.308	3
488			min	1.337	10	-30.041	2	1.184	10	0	2	-.037	1	-.207	2
489		17	max	31.097	4	143.984	3	48.064	1	0	3	.011	5	.258	3
490			min	1.337	10	-97.184	2	3.676	10	0	2	-.018	1	-.173	2
491		18	max	28.87	1	244.61	3	70.477	1	0	3	.021	4	.156	3
492			min	1.337	10	-164.326	2	5.62	12	0	2	-.001	10	-.104	2
493		19	max	28.87	1	345.235	3	92.889	1	0	3	.057	1	0	2
494			min	1.337	10	-231.468	2	6.495	12	0	2	.003	10	0	3
495	M16	1	max	35.581	5	338.809	2	5.054	5	0	3	.056	1	0	2
496			min	-29.696	1	-162.864	3	-92.641	1	0	2	-.026	5	0	3
497		2	max	30.398	5	240.349	2	6.449	5	0	3	.013	1	.074	3
498			min	-29.696	1	-115.925	3	-70.228	1	0	2	-.023	5	-.153	2
499		3	max	25.214	5	141.89	2	7.845	5	0	3	0	12	.122	3
500			min	-29.696	1	-68.987	3	-47.816	1	0	2	-.023	4	-.254	2
501		4	max	20.03	5	43.43	2	9.241	5	0	3	-.002	12	.146	3
502			min	-29.696	1	-22.048	3	-25.404	1	0	2	-.037	1	-.303	2
503		5	max	14.847	5	24.891	3	10.636	5	0	3	-.003	12	.146	3
504			min	-29.696	1	-55.029	2	-2.992	1	0	2	-.045	1	-.3	2
505		6	max	9.663	5	71.829	3	19.421	1	0	3	-.002	15	.12	3
506			min	-29.696	1	-153.488	2	-1.479	3	0	2	-.041	1	-.245	2
507		7	max	4.479	5	118.768	3	41.833	1	0	3	.003	5	.07	3
508			min	-29.696	1	-251.948	2	-.166	3	0	2	-.024	1	-.138	2
509		8	max	1.652	3	165.707	3	64.245	1	0	3	.011	4	.021	2
510			min	-29.696	1	-350.407	2	.87	12	0	2	-.006	3	-.005	3
511		9	max	1.652	3	212.645	3	86.658	1	0	3	.043	1	.232	2
512			min	-29.696	1	-448.867	2	1.745	12	0	2	-.005	3	-.105	3
513		10	max	20.711	5	-8.679	15	109.07	1	0	14	.095	1	.495	2
514			min	-29.696	1	-547.326	2	-4.734	3	0	2	-.003	3	-.23	3
515		11	max	15.527	5	448.867	2	2.904	5	0	2	.043	1	.232	2
516			min	-29.611	1	-212.645	3	-86.405	1	0	3	-.011	5	-.105	3
517		12	max	10.343	5	350.407	2	4.3	5	0	2	.005	2	.021	2
518			min	-29.611	1	-165.707	3	-63.993	1	0	3	-.009	5	-.005	3
519		13	max	5.16	5	251.948	2	5.696	5	0	2	-.001	10	.07	3
520			min	-29.611	1	-118.768	3	-41.58	1	0	3	-.025	1	-.138	2
521		14	max	.032	15	153.488	2	7.091	5	0	2	-.001	12	.12	3
522			min	-29.611	1	-71.829	3	-19.168	1	0	3	-.041	1	-.245	2
523		15	max	-1.378	10	55.029	2	9.567	4	0	2	.001	5	.146	3
524			min	-29.611	1	-24.891	3	-1.295	10	0	3	-.045	1	-.3	2
525		16	max	-1.378	10	22.048	3	25.656	1	0	2	.006	5	.146	3
526			min	-29.611	1	-43.43	2	1.197	10	0	3	-.037	1	-.303	2
527		17	max	-1.378	10	68.987	3	48.069	1	0	2	.012	5	.122	3
528			min	-29.611	1	-141.89	2	2.907	12	0	3	-.018	1	-.254	2
529		18	max	-1.378	10	115.925	3	70.481	1	0	2	.022	4	.074	3
530			min	-29.611	1	-240.349	2	3.783	12	0	3	-.001	10	-.153	2
531		19	max	-1.378	10	162.864	3	92.893	1	0	2	.057	1	0	2
532			min	-32.797	4	-338.809	2	4.658	12	0	3	.003	10	0	3
533	M15	1	max	0	1	.983	3	.102	3	0	1	0	1	0	1
534			min	-90.654	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.873	3	.102	3	0	1	0	1	0	1
536			min	-90.725	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.764	3	.102	3	0	1	0	1	0	1
538			min	-90.795	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.655	3	.102	3	0	1	0	1	0	1
540			min	-90.866	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.546	3	.102	3	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-90.936	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.437	3	.102	3	0	1	0	1	0	1
544			min	-91.007	3	0	1	0	1	0	3	0	3	-.001	3
545		7	max	0	1	.328	3	.102	3	0	1	0	3	0	1
546			min	-91.077	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.218	3	.102	3	0	1	0	3	0	1
548			min	-91.148	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.109	3	.102	3	0	1	0	3	0	1
550			min	-91.218	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.102	3	0	1	0	3	0	1
552			min	-91.289	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.102	3	0	1	0	3	0	1
554			min	-91.359	3	-.109	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.102	3	0	1	0	3	0	1
556			min	-91.43	3	-.218	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.102	3	0	1	0	3	0	1
558			min	-91.5	3	-.328	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.102	3	0	1	0	3	0	1
560			min	-91.571	3	-.437	3	0	1	0	3	0	1	-.001	3
561		15	max	0	1	0	1	.102	3	0	1	0	3	0	1
562			min	-91.641	3	-.546	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.102	3	0	1	0	3	0	1
564			min	-91.712	3	-.655	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.102	3	0	1	0	3	0	1
566			min	-91.782	3	-.764	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.102	3	0	1	0	3	0	1
568			min	-91.853	3	-.873	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.102	3	0	1	0	3	0	1
570			min	-91.923	3	-.983	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.399	4	.307	4	0	3	0	3	0	1
572			min	-188.209	4	0	2	-.042	3	0	1	0	4	0	1
573		2	max	0	2	2.132	4	.277	4	0	3	0	3	0	2
574			min	-188.212	4	0	2	-.042	3	0	1	0	4	0	4
575		3	max	0	2	1.866	4	.246	4	0	3	0	3	0	2
576			min	-188.215	4	0	2	-.042	3	0	1	0	4	-.001	4
577		4	max	0	2	1.599	4	.215	4	0	3	0	3	0	2
578			min	-188.218	4	0	2	-.042	3	0	1	0	1	-.002	4
579		5	max	0	2	1.333	4	.185	4	0	3	0	3	0	2
580			min	-188.221	4	0	2	-.042	3	0	1	0	1	-.002	4
581		6	max	0	2	1.066	4	.154	4	0	3	0	3	0	2
582			min	-188.224	4	0	2	-.042	3	0	1	0	1	-.003	4
583		7	max	0	2	.8	4	.123	4	0	3	0	5	0	2
584			min	-188.226	4	0	2	-.042	3	0	1	0	1	-.003	4
585		8	max	0	2	.533	4	.093	4	0	3	0	5	0	2
586			min	-188.229	4	0	2	-.042	3	0	1	0	1	-.003	4
587		9	max	0	2	.267	4	.062	4	0	3	0	5	0	2
588			min	-188.232	4	0	2	-.042	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.033	1	0	3	0	5	0	2
590			min	-188.235	4	0	1	-.042	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.033	1	0	3	0	5	0	2
592			min	-188.238	4	-.267	4	-.042	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.033	1	0	3	0	5	0	2
594			min	-188.241	4	-.533	4	-.042	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.033	1	0	3	0	5	0	2
596			min	-188.244	4	-.8	4	-.064	5	0	1	0	3	-.003	4
597		14	max	0	2	0	2	.033	1	0	3	0	5	0	2
598			min	-188.247	4	-1.066	4	-.095	5	0	1	0	3	-.003	4





Company : Schletter, Inc.  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	2	0	2	.033	1	0	3	0	5	0	2
600		min	-188.25	4	-1.333	4	-.126	5	0	1	0	3	-.002	4
601	16	max	.038	11	0	2	.033	1	0	3	0	1	0	2
602		min	-188.252	4	-1.599	4	-.156	5	0	1	0	3	-.002	4
603	17	max	.116	11	0	2	.033	1	0	3	0	1	0	2
604		min	-188.255	4	-1.866	4	-.187	5	0	1	0	3	-.001	4
605	18	max	.194	11	0	2	.033	1	0	3	0	1	0	2
606		min	-188.258	4	-2.132	4	-.218	5	0	1	0	5	0	4
607	19	max	.273	11	0	2	.033	1	0	3	0	1	0	1
608		min	-188.261	4	-2.399	4	-.248	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.009	2	.005	1	1.099e-3	5	NC	3	NC	2	
2			min	-.004	3	-.009	3	-.011	5	-4.562e-4	1	4187.093	2	7432.025	1	
3			2	max	.002	1	.009	2	.005	1	1.12e-3	5	NC	3	NC	2
4				min	-.003	3	-.009	3	-.011	5	-4.363e-4	1	4569.089	2	8001.506	1
5			3	max	.002	1	.008	2	.005	1	1.141e-3	5	NC	3	NC	2
6				min	-.003	3	-.009	3	-.011	5	-4.163e-4	1	5023.276	2	8674.74	1
7			4	max	.002	1	.007	2	.004	1	1.162e-3	5	NC	1	NC	2
8				min	-.003	3	-.008	3	-.01	5	-3.964e-4	1	5567.037	2	9476.474	1
9			5	max	.002	1	.006	2	.004	1	1.183e-3	5	NC	1	NC	1
10				min	-.003	3	-.008	3	-.01	5	-3.764e-4	1	6223.528	2	NC	1
11			6	max	.002	1	.006	2	.003	1	1.204e-3	5	NC	1	NC	1
12				min	-.003	3	-.008	3	-.01	5	-3.564e-4	1	7024.098	2	NC	1
13			7	max	.001	1	.005	2	.003	1	1.225e-3	5	NC	1	NC	1
14				min	-.002	3	-.007	3	-.009	5	-3.365e-4	1	8011.998	2	NC	1
15			8	max	.001	1	.004	2	.003	1	1.246e-3	5	NC	1	NC	1
16				min	-.002	3	-.007	3	-.009	5	-3.165e-4	1	9248.232	2	NC	1
17			9	max	.001	1	.004	2	.002	1	1.268e-3	5	NC	1	NC	1
18				min	-.002	3	-.006	3	-.008	5	-2.965e-4	1	NC	1	NC	1
19			10	max	.001	1	.003	2	.002	1	1.289e-3	5	NC	1	NC	1
20				min	-.002	3	-.006	3	-.008	5	-2.766e-4	1	NC	1	NC	1
21		11	max	0	1	.003	2	.002	1	1.31e-3	5	NC	1	NC	1	
22			min	-.002	3	-.005	3	-.007	5	-2.566e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.001	1	1.331e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.006	5	-2.367e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.001	1	1.352e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.006	5	-2.167e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.373e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.005	5	-1.967e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.394e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.004	5	-1.768e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.415e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.003	5	-1.568e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.436e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-1.368e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.457e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-1.169e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.478e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-9.692e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	4.576e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-6.968e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.004	5	5.745e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-7.028e-4	5	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	6.914e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-7.088e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.011	5	8.083e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-7.149e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	9.252e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-7.209e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.018	4	1.042e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-7.269e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.021	4	1.159e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-7.329e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.025	4	1.276e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-7.389e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.028	4	1.393e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	-7.45e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.031	4	1.51e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-7.51e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.035	4	1.627e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-7.57e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.038	4	1.744e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-7.63e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.041	4	1.861e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-7.691e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.044	4	1.977e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	10	-7.751e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.046	4	2.094e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-7.811e-4	5	8998.4	2	NC	1
69		16	max	.002	3	.006	2	.049	4	2.211e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-7.871e-4	5	7621.071	2	NC	1
71		17	max	.002	3	.007	2	.052	4	2.328e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-7.932e-4	5	6555.983	2	NC	1
73		18	max	.002	3	.008	2	.055	4	2.445e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-7.992e-4	5	5722.961	2	NC	1
75		19	max	.002	3	.009	2	.058	4	2.562e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-8.052e-4	5	5065.618	2	NC	1
77	M4	1	max	.001	1	.011	2	0	10	3.98e-3	5	NC	1	NC	2
78			min	0	12	-.009	3	-.061	4	-3.598e-4	1	NC	1	318.492	4
79		2	max	.001	1	.01	2	0	10	3.98e-3	5	NC	1	NC	2
80			min	0	12	-.009	3	-.056	4	-3.598e-4	1	NC	1	347.174	4
81		3	max	.001	1	.01	2	0	10	3.98e-3	5	NC	1	NC	2
82			min	0	12	-.008	3	-.051	4	-3.598e-4	1	NC	1	381.307	4
83		4	max	.001	1	.009	2	0	10	3.98e-3	5	NC	1	NC	1
84			min	0	12	-.008	3	-.046	4	-3.598e-4	1	NC	1	422.33	4
85		5	max	.001	1	.008	2	0	10	3.98e-3	5	NC	1	NC	1
86			min	0	12	-.007	3	-.041	4	-3.598e-4	1	NC	1	472.198	4
87		6	max	.001	1	.008	2	0	10	3.98e-3	5	NC	1	NC	1
88			min	0	12	-.007	3	-.036	4	-3.598e-4	1	NC	1	533.631	4
89		7	max	0	1	.007	2	0	10	3.98e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.032	4	-3.598e-4	1	NC	1	610.504	4
91		8	max	0	1	.007	2	0	10	3.98e-3	5	NC	1	NC	1
92			min	0	12	-.006	3	-.027	4	-3.598e-4	1	NC	1	708.49	4
93		9	max	0	1	.006	2	0	10	3.98e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.023	4	-3.598e-4	1	NC	1	836.168	4
95		10	max	0	1	.005	2	0	10	3.98e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.019	4	-3.598e-4	1	NC	1	1007.025	4
97		11	max	0	1	.005	2	0	10	3.98e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.016	4	-3.598e-4	1	NC	1	1243.273	4
99		12	max	0	1	.004	2	0	10	3.98e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.004	3	-.012	4	-3.598e-4	1	NC	1	1583.655	4
101		max	0	1	.004	2	0	10	3.98e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.009	4	-3.598e-4	1	NC	1	2100.838	4
103		max	0	1	.003	2	0	10	3.98e-3	5	NC	1	NC	1
104		min	0	12	-.003	3	-.007	4	-3.598e-4	1	NC	1	2944.71	4
105		max	0	1	.002	2	0	10	3.98e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.004	4	-3.598e-4	1	NC	1	4467.395	4
107		max	0	1	.002	2	0	10	3.98e-3	5	NC	1	NC	1
108		min	0	12	-.002	3	-.003	4	-3.598e-4	1	NC	1	7669.863	4
109		max	0	1	.001	2	0	10	3.98e-3	5	NC	1	NC	1
110		min	0	12	-.001	3	-.001	4	-3.598e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	3.98e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-3.598e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	3.98e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-3.598e-4	1	NC	1	NC	1
115	M6	max	.007	1	.032	2	.002	1	1.179e-3	4	NC	3	NC	1
116		min	-.011	3	-.029	3	-.011	5	-7.861e-8	2	1249.082	2	6956.694	3
117		max	.006	1	.03	2	.002	1	1.199e-3	4	NC	3	NC	1
118		min	-.011	3	-.028	3	-.011	5	-7.414e-8	2	1336.683	2	7387.216	3
119		max	.006	1	.027	2	.002	1	1.22e-3	4	NC	3	NC	1
120		min	-.01	3	-.026	3	-.011	5	-1.903e-6	1	1437.08	2	7898.388	3
121		max	.006	1	.025	2	.001	1	1.24e-3	4	NC	3	NC	1
122		min	-.009	3	-.025	3	-.011	5	-4.565e-6	1	1552.837	2	8507.429	3
123		max	.005	1	.023	2	.001	1	1.26e-3	4	NC	3	NC	1
124		min	-.009	3	-.023	3	-.01	5	-7.227e-6	1	1687.256	2	9237.18	3
125		max	.005	1	.021	2	.001	1	1.281e-3	4	NC	3	NC	1
126		min	-.008	3	-.021	3	-.01	5	-9.89e-6	1	1844.664	2	NC	1
127		max	.005	1	.019	2	.001	1	1.301e-3	4	NC	3	NC	1
128		min	-.007	3	-.02	3	-.01	5	-1.255e-5	1	2030.83	2	NC	1
129		max	.004	1	.017	2	0	1	1.322e-3	4	NC	3	NC	1
130		min	-.007	3	-.018	3	-.009	5	-1.521e-5	1	2253.628	2	NC	1
131		max	.004	1	.016	2	0	1	1.342e-3	4	NC	3	NC	1
132		min	-.006	3	-.017	3	-.009	5	-1.788e-5	1	2524.081	2	NC	1
133		max	.003	1	.014	2	0	1	1.363e-3	4	NC	3	NC	1
134		min	-.006	3	-.015	3	-.008	5	-2.054e-5	1	2858.114	2	NC	1
135		max	.003	1	.012	2	0	1	1.383e-3	4	NC	3	NC	1
136		min	-.005	3	-.013	3	-.007	5	-2.32e-5	1	3279.605	2	NC	1
137		max	.003	1	.01	2	0	1	1.403e-3	4	NC	3	NC	1
138		min	-.004	3	-.012	3	-.007	5	-2.586e-5	1	3826.07	2	NC	1
139		max	.002	1	.009	2	0	1	1.424e-3	4	NC	3	NC	1
140		min	-.004	3	-.01	3	-.006	5	-2.853e-5	1	4560.022	2	NC	1
141		max	.002	1	.007	2	0	1	1.444e-3	4	NC	3	NC	1
142		min	-.003	3	-.009	3	-.005	5	-3.119e-5	1	5593.951	2	NC	1
143		max	.002	1	.006	2	0	1	1.465e-3	4	NC	1	NC	1
144		min	-.002	3	-.007	3	-.004	5	-3.385e-5	1	7152.792	2	NC	1
145		max	.001	1	.004	2	0	1	1.485e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.003	5	-3.651e-5	1	9761.316	2	NC	1
147		max	0	1	.003	2	0	1	1.506e-3	4	NC	1	NC	1
148		min	-.001	3	-.003	3	-.002	5	-3.918e-5	1	NC	1	NC	1
149		max	0	1	.001	2	0	1	1.526e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-4.184e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.547e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-4.45e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.086e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-7.292e-4	5	NC	1	NC	1
155		max	0	3	.001	2	.004	5	1.819e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-7.234e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.007	5	1.553e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-7.18e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.011	5	1.287e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-7.126e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.015	5	1.021e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-7.072e-4	4	8402.474	2	NC	1
163		6	max	.002	3	.007	2	.019	4	2.409e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	-7.018e-4	4	6733.343	2	NC	1
165		7	max	.002	3	.008	2	.022	4	4.716e-5	3	NC	3	NC	1
166			min	-.002	2	-.011	3	0	1	-6.963e-4	4	5592.85	2	NC	1
167		8	max	.002	3	.01	2	.026	4	7.022e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-6.909e-4	4	4757.461	2	NC	1
169		9	max	.003	3	.011	2	.029	4	9.329e-5	3	NC	3	NC	1
170			min	-.003	2	-.015	3	0	1	-6.855e-4	4	4115.778	2	NC	1
171		10	max	.003	3	.013	2	.032	4	1.163e-4	3	NC	3	NC	1
172			min	-.004	2	-.016	3	0	1	-6.801e-4	4	3606.045	2	NC	1
173		11	max	.003	3	.014	2	.036	4	1.394e-4	3	NC	3	NC	1
174			min	-.004	2	-.018	3	0	1	-6.747e-4	4	3191.175	2	NC	1
175		12	max	.004	3	.016	2	.039	4	1.625e-4	3	NC	3	NC	1
176			min	-.004	2	-.019	3	0	1	-6.693e-4	4	2847.442	2	NC	1
177		13	max	.004	3	.018	2	.042	4	1.855e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	0	1	-6.639e-4	4	2558.846	2	NC	1
179		14	max	.004	3	.02	2	.045	4	2.086e-4	3	NC	3	NC	1
180			min	-.005	2	-.021	3	0	1	-6.584e-4	4	2314.12	2	NC	1
181		15	max	.005	3	.022	2	.048	4	2.317e-4	3	NC	3	NC	1
182			min	-.006	2	-.022	3	0	1	-6.53e-4	4	2105.03	2	NC	1
183		16	max	.005	3	.024	2	.051	4	2.547e-4	3	NC	3	NC	1
184			min	-.006	2	-.023	3	0	1	-6.476e-4	4	1925.374	2	NC	1
185		17	max	.005	3	.026	2	.053	4	2.778e-4	3	NC	3	NC	1
186			min	-.006	2	-.024	3	-.001	1	-6.422e-4	4	1770.364	2	NC	1
187		18	max	.006	3	.028	2	.056	4	3.009e-4	3	NC	3	NC	1
188			min	-.007	2	-.025	3	-.001	1	-6.368e-4	4	1636.232	2	NC	1
189		19	max	.006	3	.03	2	.059	4	3.239e-4	3	NC	3	NC	1
190			min	-.007	2	-.026	3	-.001	1	-6.314e-4	4	1519.966	2	NC	1
191	M8	1	max	.004	1	.036	2	.001	1	3.81e-3	4	NC	1	NC	1
192			min	0	3	-.029	3	-.061	4	-2.447e-4	3	NC	1	314.435	4
193		2	max	.004	1	.034	2	0	1	3.81e-3	4	NC	1	NC	1
194			min	0	3	-.027	3	-.056	4	-2.447e-4	3	NC	1	342.75	4
195		3	max	.004	1	.032	2	0	1	3.81e-3	4	NC	1	NC	1
196			min	0	3	-.026	3	-.051	4	-2.447e-4	3	NC	1	376.45	4
197		4	max	.003	1	.03	2	0	1	3.81e-3	4	NC	1	NC	1
198			min	0	3	-.024	3	-.046	4	-2.447e-4	3	NC	1	416.95	4
199		5	max	.003	1	.028	2	0	1	3.81e-3	4	NC	1	NC	1
200			min	0	3	-.023	3	-.041	4	-2.447e-4	3	NC	1	466.184	4
201		6	max	.003	1	.026	2	0	1	3.81e-3	4	NC	1	NC	1
202			min	0	3	-.021	3	-.037	4	-2.447e-4	3	NC	1	526.836	4
203		7	max	.003	1	.024	2	0	1	3.81e-3	4	NC	1	NC	1
204			min	0	3	-.019	3	-.032	4	-2.447e-4	3	NC	1	602.731	4
205		8	max	.002	1	.022	2	0	1	3.81e-3	4	NC	1	NC	1
206			min	0	3	-.018	3	-.028	4	-2.447e-4	3	NC	1	699.471	4
207		9	max	.002	1	.02	2	0	1	3.81e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.023	4	-2.447e-4	3	NC	1	825.526	4
209		10	max	.002	1	.018	2	0	1	3.81e-3	4	NC	1	NC	1
210			min	0	3	-.015	3	-.019	4	-2.447e-4	3	NC	1	994.212	4
211		11	max	.002	1	.016	2	0	1	3.81e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.016	4	-2.447e-4	3	NC	1	1227.459	4
213		12	max	.002	1	.014	2	0	1	3.81e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.011	3	-.012	4	-2.447e-4	3	NC	1	1563.516	4
215		13	max	.001	1	.012	2	0	1	3.81e-3	4	NC	1	NC	1
216			min	0	3	-.01	3	-.009	4	-2.447e-4	3	NC	1	2074.131	4
217		14	max	.001	1	.01	2	0	1	3.81e-3	4	NC	1	NC	1
218			min	0	3	-.008	3	-.007	4	-2.447e-4	3	NC	1	2907.287	4
219		15	max	0	1	.008	2	0	1	3.81e-3	4	NC	1	NC	1
220			min	0	3	-.006	3	-.004	4	-2.447e-4	3	NC	1	4410.64	4
221		16	max	0	1	.006	2	0	1	3.81e-3	4	NC	1	NC	1
222			min	0	3	-.005	3	-.003	4	-2.447e-4	3	NC	1	7572.455	4
223		17	max	0	1	.004	2	0	1	3.81e-3	4	NC	1	NC	1
224			min	0	3	-.003	3	-.001	4	-2.447e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	3.81e-3	4	NC	1	NC	1
226			min	0	3	-.002	3	0	4	-2.447e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	3.81e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.447e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.009	2	0	3	4.573e-4	1	NC	3	NC	1
230			min	-.003	3	-.009	3	-.005	4	-5.11e-4	3	4191.678	2	NC	1
231		2	max	.002	1	.009	2	0	3	4.34e-4	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.005	4	-4.937e-4	3	4574.233	2	NC	1
233		3	max	.002	1	.008	2	0	3	4.108e-4	1	NC	3	NC	1
234			min	-.003	3	-.009	3	-.006	4	-4.763e-4	3	5029.113	2	NC	1
235		4	max	.002	1	.007	2	0	3	4.024e-4	4	NC	1	NC	1
236			min	-.003	3	-.008	3	-.006	4	-4.59e-4	3	5573.739	2	NC	1
237		5	max	.002	1	.006	2	0	3	4.573e-4	4	NC	1	NC	1
238			min	-.002	3	-.008	3	-.006	4	-4.416e-4	3	6231.32	2	NC	1
239		6	max	.002	1	.006	2	0	3	5.123e-4	4	NC	1	NC	1
240			min	-.002	3	-.008	3	-.006	4	-4.243e-4	3	7033.283	2	NC	1
241		7	max	.001	1	.005	2	0	3	5.673e-4	4	NC	1	NC	1
242			min	-.002	3	-.007	3	-.006	4	-4.069e-4	3	8022.989	2	NC	1
243		8	max	.001	1	.004	2	0	3	6.222e-4	4	NC	1	NC	1
244			min	-.002	3	-.007	3	-.006	4	-3.896e-4	3	9261.604	2	NC	1
245		9	max	.001	1	.004	2	0	3	6.772e-4	4	NC	1	NC	1
246			min	-.002	3	-.006	3	-.006	4	-3.722e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	7.322e-4	4	NC	1	NC	1
248			min	-.002	3	-.006	3	-.005	4	-3.549e-4	3	NC	1	NC	1
249		11	max	0	1	.003	2	0	3	7.871e-4	4	NC	1	NC	1
250			min	-.001	3	-.005	3	-.005	4	-3.375e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	8.421e-4	4	NC	1	NC	1
252			min	-.001	3	-.005	3	-.005	4	-3.202e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	8.971e-4	4	NC	1	NC	1
254			min	-.001	3	-.004	3	-.004	4	-3.028e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	9.52e-4	4	NC	1	NC	1
256			min	0	3	-.003	3	-.004	4	-2.855e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.007e-3	4	NC	1	NC	1
258			min	0	3	-.003	3	-.003	4	-2.681e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.062e-3	4	NC	1	NC	1
260			min	0	3	-.002	3	-.002	4	-2.508e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.117e-3	4	NC	1	NC	1
262			min	0	3	-.001	3	-.002	4	-2.334e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.172e-3	4	NC	1	NC	1
264			min	0	3	0	3	0	4	-2.161e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.227e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.987e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	9.38e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-5.788e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.003	4	7.095e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-6.334e-4	4	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	4.81e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.88e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.525e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-7.426e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.012	4	2.404e-6	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-7.972e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.015	5	-4.943e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-8.519e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.018	5	-5.633e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-9.065e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.021	5	-6.323e-6	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-9.611e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.024	5	-7.013e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-1.016e-3	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.027	5	-7.702e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.07e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.03	5	-8.392e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-1.125e-3	4	NC	1	NC	1
289		12	max	.001	3	.003	2	.033	5	-9.082e-6	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.003	3	-1.18e-3	4	NC	1	NC	1
291		13	max	.001	3	.003	2	.035	5	-9.772e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-1.234e-3	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.038	5	-1.046e-5	10	NC	1	NC	1
294			min	-.001	2	-.008	3	-.003	1	-1.289e-3	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.041	5	-1.115e-5	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.004	1	-1.343e-3	4	9012.154	2	NC	1
297		16	max	.002	3	.006	2	.043	5	-1.184e-5	10	NC	1	NC	1
298			min	-.002	2	-.008	3	-.004	1	-1.398e-3	4	7631.593	2	NC	1
299		17	max	.002	3	.007	2	.046	5	-1.253e-5	10	NC	1	NC	1
300			min	-.002	2	-.008	3	-.004	1	-1.453e-3	4	6564.253	2	NC	1
301		18	max	.002	3	.008	2	.049	5	-1.322e-5	10	NC	1	NC	2
302			min	-.002	2	-.008	3	-.005	1	-1.507e-3	4	5729.631	2	9485.301	1
303		19	max	.002	3	.009	2	.051	5	-1.391e-5	10	NC	3	NC	2
304			min	-.002	2	-.008	3	-.005	1	-1.562e-3	4	5071.133	2	8757.046	1
305	M12	1	max	.001	1	.011	2	.004	1	4.646e-3	4	NC	1	NC	2
306			min	0	15	-.009	3	-.056	5	1.538e-5	10	NC	1	342.216	5
307		2	max	.001	1	.01	2	.004	1	4.646e-3	4	NC	1	NC	2
308			min	0	15	-.009	3	-.052	5	1.538e-5	10	NC	1	373.025	5
309		3	max	.001	1	.01	2	.004	1	4.646e-3	4	NC	1	NC	2
310			min	0	15	-.008	3	-.047	5	1.538e-5	10	NC	1	409.691	5
311		4	max	.001	1	.009	2	.003	1	4.646e-3	4	NC	1	NC	2
312			min	0	15	-.008	3	-.043	5	1.538e-5	10	NC	1	453.756	5
313		5	max	.001	1	.008	2	.003	1	4.646e-3	4	NC	1	NC	2
314			min	0	15	-.007	3	-.038	5	1.538e-5	10	NC	1	507.32	5
315		6	max	.001	1	.008	2	.003	1	4.646e-3	4	NC	1	NC	2
316			min	0	15	-.007	3	-.034	5	1.538e-5	10	NC	1	573.306	5
317		7	max	0	1	.007	2	.002	1	4.646e-3	4	NC	1	NC	2
318			min	0	15	-.006	3	-.029	5	1.538e-5	10	NC	1	655.875	5
319		8	max	0	1	.007	2	.002	1	4.646e-3	4	NC	1	NC	2
320			min	0	15	-.006	3	-.025	5	1.538e-5	10	NC	1	761.12	5
321		9	max	0	1	.006	2	.002	1	4.646e-3	4	NC	1	NC	1
322			min	0	15	-.005	3	-.022	5	1.538e-5	10	NC	1	898.253	5
323		10	max	0	1	.005	2	.001	1	4.646e-3	4	NC	1	NC	1
324			min	0	15	-.005	3	-.018	5	1.538e-5	10	NC	1	1081.761	5
325		11	max	0	1	.005	2	.001	1	4.646e-3	4	NC	1	NC	1
326			min	0	15	-.004	3	-.014	5	1.538e-5	10	NC	1	1335.497	5
327		12	max	0	1	.004	2	0	1	4.646e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	15	-.004	3	-.011	5	1.538e-5	10	NC	1	1701.068	5
329		13	max	0	1	.004	2	0	1	4.646e-3	4	NC	1	NC	1
330			min	0	15	-.003	3	-.009	5	1.538e-5	10	NC	1	2256.515	5
331		14	max	0	1	.003	2	0	1	4.646e-3	4	NC	1	NC	1
332			min	0	15	-.003	3	-.006	5	1.538e-5	10	NC	1	3162.805	5
333		15	max	0	1	.002	2	0	1	4.646e-3	4	NC	1	NC	1
334			min	0	15	-.002	3	-.004	5	1.538e-5	10	NC	1	4798.085	5
335		16	max	0	1	.002	2	0	1	4.646e-3	4	NC	1	NC	1
336			min	0	15	-.002	3	-.002	5	1.538e-5	10	NC	1	8237.293	5
337		17	max	0	1	.001	2	0	1	4.646e-3	4	NC	1	NC	1
338			min	0	15	-.001	3	-.001	5	1.538e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.646e-3	4	NC	1	NC	1
340			min	0	15	0	3	0	5	1.538e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.646e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	1.538e-5	10	NC	1	NC	1
343	M1	1	max	.008	3	.025	3	.006	5	7.999e-3	2	NC	1	NC	1
344			min	-.009	2	-.021	2	-.002	1	-1.154e-2	3	NC	1	NC	1
345		2	max	.008	3	.015	3	.009	5	3.931e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.004	1	-5.697e-3	3	4685.091	3	NC	1
347		3	max	.008	3	.005	3	.011	5	3.541e-4	5	NC	4	NC	1
348			min	-.009	2	-.004	2	-.005	1	-2.641e-4	1	2427.145	3	9678.171	5
349		4	max	.008	3	.004	2	.014	5	3.588e-4	5	NC	4	NC	1
350			min	-.009	2	-.003	3	-.006	1	-2.251e-4	1	1729.828	3	6072.376	5
351		5	max	.008	3	.01	2	.017	5	3.634e-4	5	NC	4	NC	1
352			min	-.009	2	-.009	3	-.006	1	-1.861e-4	1	1397.402	3	4324.76	5
353		6	max	.008	3	.016	2	.021	5	3.68e-4	5	NC	4	NC	1
354			min	-.009	2	-.015	3	-.006	1	-1.471e-4	1	1208.644	2	3310.085	5
355		7	max	.008	3	.02	2	.024	5	3.727e-4	5	NC	4	NC	1
356			min	-.009	2	-.019	3	-.005	1	-1.08e-4	1	1075.684	2	2656.255	5
357		8	max	.008	3	.023	2	.028	5	3.773e-4	5	NC	4	NC	1
358			min	-.009	2	-.022	3	-.004	1	-6.901e-5	1	992.044	2	2205.131	5
359		9	max	.008	3	.025	2	.031	5	3.819e-4	5	NC	4	NC	1
360			min	-.009	2	-.023	3	-.003	1	-3.07e-5	9	942.142	2	1878.462	5
361		10	max	.008	3	.026	2	.035	5	3.89e-4	4	NC	4	NC	1
362			min	-.009	2	-.024	3	-.002	1	-3.23e-6	9	918.516	2	1614.994	4
363		11	max	.008	3	.026	2	.039	4	4.03e-4	4	NC	4	NC	1
364			min	-.009	2	-.023	3	0	9	5.003e-6	10	918.439	2	1415.297	4
365		12	max	.008	3	.024	2	.043	4	4.17e-4	4	NC	4	NC	1
366			min	-.009	2	-.021	3	0	10	6.638e-6	10	942.896	2	1261.064	4
367		13	max	.008	3	.021	2	.047	4	4.309e-4	4	NC	4	NC	1
368			min	-.009	2	-.018	3	0	10	8.273e-6	10	997.159	2	1139.959	4
369		14	max	.008	3	.017	2	.051	4	4.449e-4	4	NC	4	NC	1
370			min	-.009	2	-.014	3	0	10	9.909e-6	10	1093.474	2	1043.737	4
371		15	max	.008	3	.011	2	.054	4	4.589e-4	4	NC	4	NC	1
372			min	-.009	2	-.009	3	0	10	1.154e-5	10	1258.798	2	966.735	4
373		16	max	.008	3	.003	2	.058	4	6.825e-4	4	NC	4	NC	1
374			min	-.009	2	-.003	3	0	10	1.275e-5	10	1559.259	2	904.967	4
375		17	max	.008	3	.005	3	.061	4	5.895e-3	4	NC	4	NC	1
376			min	-.009	2	-.006	2	0	10	-2.553e-5	9	2207.903	2	855.642	4
377		18	max	.008	3	.012	3	.063	4	5.738e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	10	-2.88e-3	3	4278.448	2	816.594	4
379		19	max	.008	3	.021	3	.065	4	1.156e-2	2	NC	1	NC	1
380			min	-.009	2	-.028	2	-.001	1	-5.871e-3	3	NC	1	787.246	4
381	M5	1	max	.026	3	.08	3	.006	5	1.38e-5	4	NC	1	NC	1
382			min	-.029	2	-.069	2	-.002	1	5.561e-8	11	NC	1	NC	1
383		2	max	.026	3	.047	3	.009	5	1.769e-4	5	NC	4	NC	1
384			min	-.029	2	-.04	2	-.002	1	-3.222e-5	1	1467.753	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.026	3	.017	3	.011	5	3.374e-4	5	NC	5	NC	1
386		min	-.029	2	-.013	2	-.002	1	-6.39e-5	1	760.688	3	NC	1
387	4	max	.026	3	.012	2	.014	5	3.518e-4	5	NC	5	NC	1
388		min	-.029	2	-.009	3	-.002	1	-6.065e-5	1	542.675	3	NC	1
389	5	max	.026	3	.033	2	.018	5	3.662e-4	5	NC	5	NC	1
390		min	-.029	2	-.03	3	-.002	1	-5.74e-5	1	437.347	2	NC	1
391	6	max	.026	3	.051	2	.021	5	3.805e-4	5	NC	5	NC	1
392		min	-.029	2	-.047	3	-.002	1	-5.416e-5	1	371.018	2	9271.772	3
393	7	max	.026	3	.065	2	.025	5	3.949e-4	5	NC	5	NC	1
394		min	-.029	2	-.06	3	-.002	1	-5.091e-5	1	330.03	2	8797.604	3
395	8	max	.026	3	.076	2	.029	5	4.093e-4	5	NC	5	NC	1
396		min	-.029	2	-.069	3	-.002	1	-4.766e-5	1	304.228	2	8681.295	3
397	9	max	.026	3	.083	2	.033	5	4.237e-4	5	NC	5	NC	1
398		min	-.029	2	-.074	3	-.002	1	-4.441e-5	1	288.809	2	8853.135	3
399	10	max	.026	3	.086	2	.037	5	4.38e-4	5	NC	5	NC	1
400		min	-.029	2	-.075	3	-.001	1	-4.117e-5	1	281.472	2	9303.089	3
401	11	max	.025	3	.084	2	.041	5	4.524e-4	5	NC	5	NC	1
402		min	-.029	2	-.073	3	-.001	1	-3.792e-5	1	281.372	2	NC	1
403	12	max	.025	3	.079	2	.045	5	4.668e-4	5	NC	5	NC	1
404		min	-.029	2	-.067	3	-.001	1	-3.467e-5	1	288.806	2	NC	1
405	13	max	.025	3	.069	2	.049	5	4.812e-4	5	NC	5	NC	1
406		min	-.029	2	-.057	3	-.001	1	-3.165e-5	9	305.388	2	NC	1
407	14	max	.025	3	.054	2	.052	4	4.955e-4	5	NC	5	NC	1
408		min	-.029	2	-.044	3	-.001	1	-2.906e-5	9	334.875	2	NC	1
409	15	max	.025	3	.034	2	.056	4	5.099e-4	5	NC	5	NC	1
410		min	-.029	2	-.028	3	-.001	1	-2.646e-5	9	385.541	2	NC	1
411	16	max	.025	3	.01	2	.059	4	7.311e-4	5	NC	5	NC	1
412		min	-.029	2	-.008	3	-.001	1	-2.568e-5	9	477.714	2	NC	1
413	17	max	.025	3	.014	3	.061	4	5.892e-3	4	NC	5	NC	1
414		min	-.029	2	-.021	2	-.001	1	-8.058e-5	1	677.139	2	NC	1
415	18	max	.025	3	.039	3	.064	4	3.024e-3	4	NC	4	NC	1
416		min	-.029	2	-.055	2	0	1	-4.123e-5	1	1312.864	2	NC	1
417	19	max	.025	3	.065	3	.065	4	4.266e-6	5	NC	1	NC	1
418		min	-.029	2	-.092	2	0	1	-8.94e-7	3	NC	1	NC	1
419	M9	1	max	.009	.025	.006	.006	5	1.155e-2	3	NC	1	NC	1
420		min	-.009	2	-.021	2	-.002	1	-7.998e-3	2	NC	1	NC	1
421	2	max	.009	3	.014	.005	.005	5	5.692e-3	3	NC	4	NC	1
422		min	-.009	2	-.012	2	0	9	-3.931e-3	2	4686.741	3	NC	1
423	3	max	.009	3	.005	.005	.005	4	1.222e-4	1	NC	4	NC	1
424		min	-.009	2	-.004	2	0	3	-5.777e-5	3	2428.015	3	NC	1
425	4	max	.009	3	.004	.006	.006	4	8.903e-5	1	NC	4	NC	1
426		min	-.009	2	-.003	3	-.001	3	-6.272e-5	3	1730.426	3	NC	1
427	5	max	.008	3	.01	.008	.008	4	5.581e-5	1	NC	4	NC	1
428		min	-.009	2	-.01	3	-.002	3	-6.766e-5	3	1397.842	3	NC	1
429	6	max	.008	3	.016	.01	.01	4	2.692e-5	2	NC	4	NC	1
430		min	-.009	2	-.015	3	-.003	3	-7.26e-5	3	1208.969	2	8784.122	3
431	7	max	.008	3	.02	.013	.013	4	1.555e-5	2	NC	4	NC	1
432		min	-.009	2	-.019	3	-.004	3	-7.754e-5	3	1075.984	2	6041.988	4
433	8	max	.008	3	.023	.016	.016	4	7.551e-6	5	NC	4	NC	1
434		min	-.009	2	-.022	3	-.004	3	-8.249e-5	3	992.332	2	4309.273	4
435	9	max	.008	3	.025	.019	.019	4	1.789e-5	5	NC	4	NC	1
436		min	-.009	2	-.024	3	-.005	3	-8.743e-5	3	942.425	2	3256.242	4
437	10	max	.008	3	.026	.023	.023	5	2.824e-5	5	NC	4	NC	1
438		min	-.009	2	-.024	3	-.005	3	-1.103e-4	1	918.801	2	2567.131	4
439	11	max	.008	3	.026	.028	.028	5	3.858e-5	5	NC	4	NC	1
440		min	-.009	2	-.023	3	-.005	3	-1.435e-4	1	918.732	2	2090.807	4
441	12	max	.008	3	.024	.032	.032	5	4.893e-5	5	NC	4	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.006	4	-.018	1	-.04	3	-5.001e-3	2	3682.58	2	1165.61	3
557	13	max	.001	3	.005	5	.031	1	5.841e-3	3	NC	5	4737.86	15
558		min	-.006	4	-.017	1	-.039	3	-5.45e-3	2	3988.041	2	1154.581	3
559	14	max	.001	3	.005	5	.028	1	6.295e-3	3	NC	5	6399.695	15
560		min	-.007	4	-.015	1	-.035	3	-5.899e-3	2	4497.015	2	1191.042	3
561	15	max	.001	3	.006	5	.023	1	6.748e-3	3	NC	5	9995.186	15
562		min	-.007	4	-.012	9	-.029	3	-6.349e-3	2	5343.375	2	1293.574	3
563	16	max	.001	3	.006	5	.016	1	7.202e-3	3	NC	5	NC	5
564		min	-.008	4	-.01	9	-.019	3	-6.798e-3	2	6847.758	2	1512.532	3
565	17	max	.001	3	.006	5	.007	1	7.655e-3	3	NC	3	NC	4
566		min	-.008	4	-.007	9	-.006	3	-7.247e-3	2	9981.305	2	2005.825	3
567	18	max	.002	3	.006	5	.01	3	8.109e-3	3	NC	1	NC	4
568		min	-.009	4	-.005	9	-.013	2	-7.696e-3	2	NC	1	3572.166	3
569	19	max	.002	3	.006	2	.032	3	8.562e-3	3	NC	1	NC	1
570		min	-.01	4	-.002	9	-.031	2	-8.146e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.01	3	2.46e-3	3	NC	1	NC	1
572		min	-.003	4	-.004	4	-.01	2	-2.422e-3	2	NC	1	NC	1
573	2	max	0	10	-.002	10	.002	3	2.365e-3	3	NC	1	NC	1
574		min	-.003	4	-.01	4	-.004	2	-2.314e-3	2	NC	1	NC	1
575	3	max	0	10	-.004	12	.005	1	2.27e-3	3	NC	3	NC	4
576		min	-.003	4	-.016	4	-.007	5	-2.205e-3	2	5451.098	4	5706.896	3
577	4	max	0	10	-.006	12	.008	1	2.176e-3	3	NC	12	NC	9
578		min	-.003	4	-.022	4	-.012	5	-2.097e-3	2	3739.771	4	4345.748	3
579	5	max	0	10	-.007	12	.011	1	2.081e-3	3	NC	12	NC	9
580		min	-.003	4	-.027	4	-.018	5	-1.988e-3	2	2918.182	4	3758.387	3
581	6	max	0	10	-.008	12	.012	1	1.986e-3	3	8994.03	12	NC	14
582		min	-.002	4	-.031	4	-.025	5	-1.88e-3	2	2455.958	4	2944.499	5
583	7	max	0	10	-.009	12	.013	1	1.891e-3	3	7976.082	12	9617.826	9
584		min	-.002	4	-.034	4	-.031	5	-1.771e-3	2	2177.992	4	2340.152	5
585	8	max	0	10	-.01	12	.013	1	1.796e-3	3	7365.16	12	9705.817	9
586		min	-.002	4	-.037	4	-.036	5	-1.663e-3	2	2011.17	4	1994.419	5
587	9	max	0	10	-.01	12	.012	1	1.702e-3	3	7036.326	12	NC	9
588		min	-.002	4	-.038	4	-.04	5	-1.555e-3	2	1921.377	4	1797.324	5
589	10	max	0	10	-.01	12	.011	1	1.607e-3	3	6932.298	12	NC	9
590		min	-.002	4	-.038	4	-.042	5	-1.446e-3	2	1892.971	4	1699.487	5
591	11	max	0	10	-.01	12	.009	1	1.512e-3	3	7036.326	12	NC	9
592		min	-.002	4	-.038	4	-.042	5	-1.338e-3	2	1921.377	4	1680.019	5
593	12	max	0	10	-.01	12	.008	1	1.417e-3	3	7365.16	12	NC	9
594		min	-.001	4	-.036	4	-.041	5	-1.229e-3	2	2011.17	4	1735.559	5
595	13	max	0	10	-.009	12	.006	1	1.323e-3	3	7976.082	12	NC	2
596		min	-.001	4	-.033	4	-.037	5	-1.121e-3	2	2177.992	4	1878.941	5
597	14	max	0	10	-.008	12	.004	1	1.228e-3	3	8994.03	12	NC	1
598		min	0	4	-.029	4	-.033	5	-1.012e-3	2	2455.958	4	2146.497	5
599	15	max	0	10	-.007	12	.003	1	1.133e-3	3	NC	12	NC	1
600		min	0	4	-.025	4	-.027	5	-9.038e-4	2	2918.182	4	2622.495	5
601	16	max	0	10	-.005	12	.001	9	1.038e-3	3	NC	12	NC	1
602		min	0	4	-.019	4	-.02	5	-7.953e-4	2	3739.771	4	3517.515	5
603	17	max	0	10	-.004	12	0	9	9.434e-4	3	NC	3	NC	1
604		min	0	4	-.013	4	-.013	5	-6.868e-4	2	5451.098	4	5489.745	5
605	18	max	0	10	-.002	12	0	3	9.416e-4	4	NC	1	NC	1
606		min	0	4	-.007	4	-.006	5	-5.784e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	1.007e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.699e-4	2	NC	1	NC	1



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E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.