



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

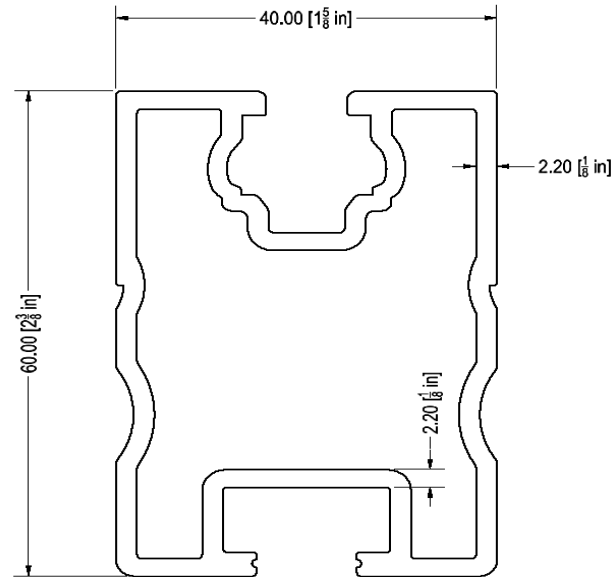
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39 in
ΦF_{ty} STRONG-AXIS =	30.12 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	-0.296 k-ft
M_z =	-0.022 k-ft
$M_{y \text{ allowable}}$ =	1.281 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	26%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

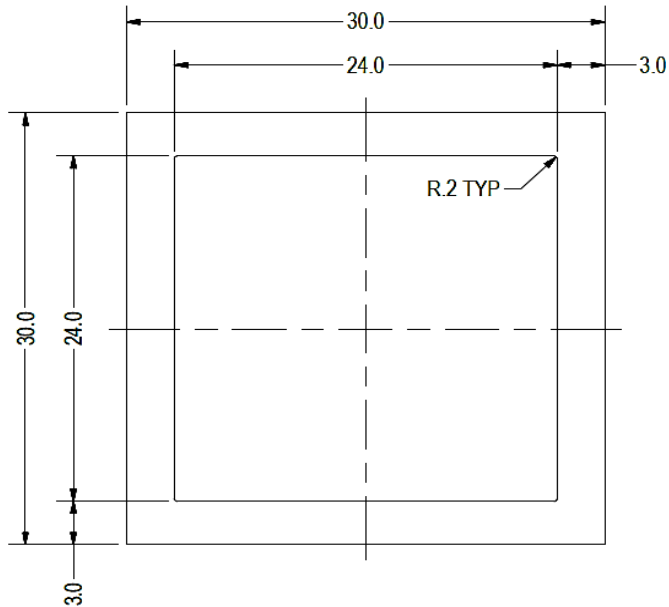
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.66 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.533 k-ft
M_z =	0.000 k-ft
P_n =	0.249 k
$M_{y \text{ allowable}}$ =	1.455 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	39%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

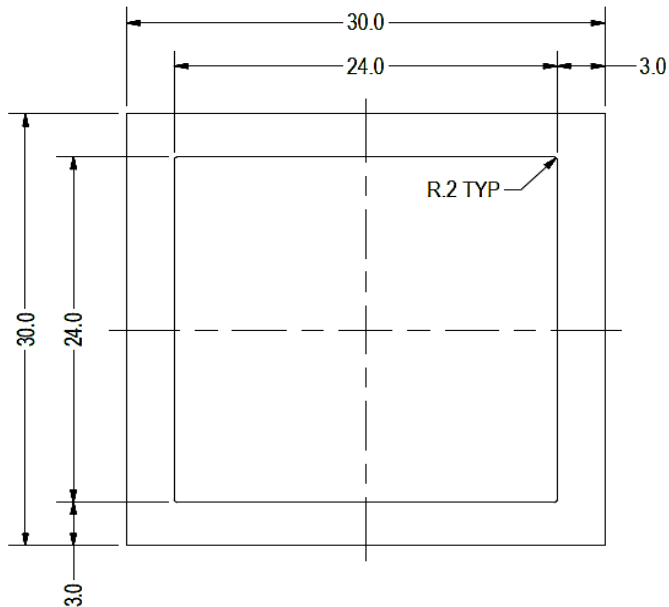
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.021 k-ft
P_n =	0.111 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.690 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	18%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.557 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	13%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.167 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

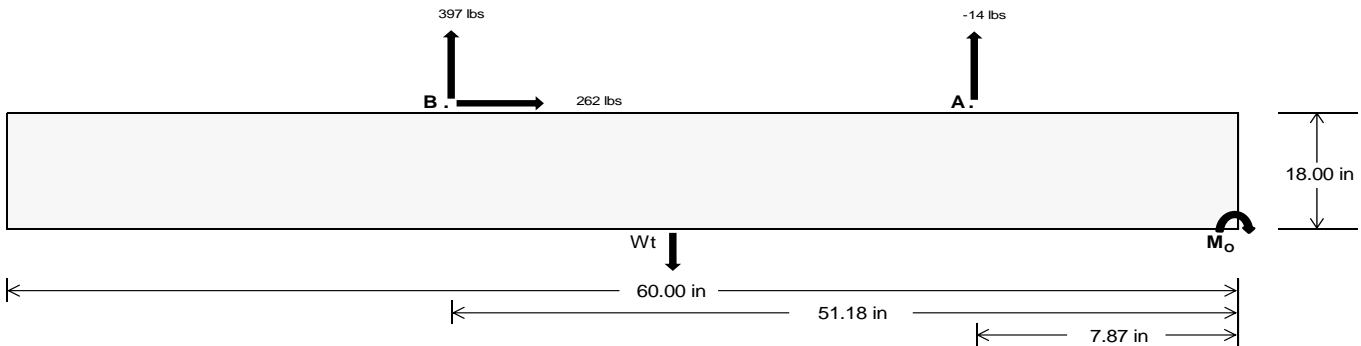
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	1.31	1724.78	k
Compressive Load =	745.85	1099.01	k
Lateral Load =	17.83	1136.90	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24951.9$ in-lbs
Resisting Force Required = 831.73 lbs
S.F. = 1.67
Weight Required = 1386.22 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 262.29 lbs
Friction = 0.4
Weight Required = 655.73 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 262.29 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	240 lbs	240 lbs	240 lbs	240 lbs	302 lbs	302 lbs	302 lbs	302 lbs	381 lbs	381 lbs	381 lbs	381 lbs	29 lbs	29 lbs	29 lbs	29 lbs
F_B	149 lbs	149 lbs	149 lbs	149 lbs	481 lbs	481 lbs	481 lbs	481 lbs	457 lbs	457 lbs	457 lbs	457 lbs	-795 lbs	-795 lbs	-795 lbs	-795 lbs
F_V	18 lbs	18 lbs	18 lbs	18 lbs	470 lbs	470 lbs	470 lbs	470 lbs	364 lbs	364 lbs	364 lbs	364 lbs	-525 lbs	-525 lbs	-525 lbs	-525 lbs
P_{total}	2202 lbs	2293 lbs	2384 lbs	2474 lbs	2595 lbs	2686 lbs	2777 lbs	2867 lbs	2650 lbs	2741 lbs	2832 lbs	2922 lbs	321 lbs	376 lbs	430 lbs	484 lbs
M	207 lbs-ft	207 lbs-ft	207 lbs-ft	207 lbs-ft	411 lbs-ft	411 lbs-ft	411 lbs-ft	411 lbs-ft	444 lbs-ft	444 lbs-ft	444 lbs-ft	444 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft
e	0.09 ft	0.09 ft	0.09 ft	0.08 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	2.08 ft	1.78 ft	1.56 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	234.5 psf	233.7 psf	233.0 psf	232.3 psf	252.3 psf	250.6 psf	249.1 psf	247.8 psf	254.2 psf	252.4 psf	250.8 psf	249.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	294.0 psf	290.4 psf	287.1 psf	284.1 psf	370.6 psf	363.3 psf	356.7 psf	350.6 psf	381.9 psf	374.1 psf	367.0 psf	360.5 psf	308.4 psf	199.2 psf	165.7 psf	150.6 psf

Maximum Bearing Pressure = 382 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

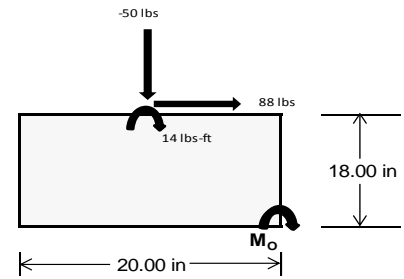
Overturning Check

$M_o = 186.6 \text{ ft-lbs}$
 Resisting Force Required = 223.91 lbs
 S.F. = 1.67
 Weight Required = 373.19 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	119 lbs	24 lbs	51 lbs	180 lbs	268 lbs	128 lbs	86 lbs	-50 lbs	20 lbs
F_v	11 lbs	88 lbs	11 lbs	8 lbs	66 lbs	8 lbs	11 lbs	88 lbs	11 lbs
P_{total}	2362 lbs	2268 lbs	2295 lbs	2316 lbs	2404 lbs	2264 lbs	742 lbs	607 lbs	676 lbs
M	29 lbs-ft	145 lbs-ft	30 lbs-ft	22 lbs-ft	110 lbs-ft	23 lbs-ft	29 lbs-ft	145 lbs-ft	30 lbs-ft
e	0.01 ft	0.06 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.24 ft	0.04 ft
$L/6$	0.28 ft	1.54 ft	1.64 ft	1.65 ft	1.58 ft	1.65 ft	1.59 ft	1.19 ft	1.58 ft
f_{min}	270.8 sqft	209.3 sqft	262.4 sqft	268.4 sqft	241.2 sqft	261.7 sqft	76.3 sqft	10.0 sqft	68.2 sqft
f_{max}	296.2 psf	334.9 psf	288.3 psf	287.3 psf	335.9 psf	281.8 psf	101.8 psf	135.5 psf	94.1 psf



Maximum Bearing Pressure = 336 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.912 k
Allowable Uplift =	1.214 k
Utilization =	<u>75%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.064 k
Allowable Uplift =	1.116 k
Utilization =	<u>95%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.574 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>

Diagonal Strut

Maximum Axial Load =	0.690 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>



Rear Strut

Maximum Axial Load =	1.008 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Bracing

Maximum Axial Load =	0.167 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.044 in
	<u>0.044 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 39.00 \text{ in}$$

$$J = 0.255$$

$$101.554$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.00 \text{ in}$$

$$J = 0.255$$

$$105.457$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.1 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.281 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

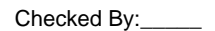
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 35° 160mph 30psf 3.25ft 7-10Pa Page 20



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	187.987	2	-.031	15	.047	1	0	10	0	4	0	15
30		min	-360.204	3	-.128	4	-.653	5	0	4	0	3	0	6
31	16	max	188.122	2	-.044	15	.047	1	0	10	0	4	0	15
32		min	-360.102	3	-.185	4	-.776	5	0	4	0	3	0	6
33	17	max	188.257	2	-.058	15	.047	1	0	10	0	4	0	15
34		min	-360.001	3	-.243	4	-.899	5	0	4	0	3	0	6
35	18	max	188.392	2	-.071	15	.047	1	0	10	0	9	0	15
36		min	-359.9	3	-.3	4	-1.023	5	0	4	0	3	0	6
37	19	max	188.527	2	-.085	15	.047	1	0	10	0	9	0	15
38		min	-359.799	3	-.358	4	-1.146	5	0	4	0	3	0	6
39	M3	1	max	242.628	2	1.734	.015	10	0	5	0	4	0	6
40		min	-224.724	3	.407	15	-1.305	4	0	1	0	10	0	15
41	2	max	242.558	2	1.558	6	.015	10	0	5	0	1	0	2
42		min	-224.776	3	.365	15	-1.172	4	0	1	0	10	0	3
43	3	max	242.488	2	1.382	6	.015	10	0	5	0	1	0	2
44		min	-224.829	3	.324	15	-1.038	4	0	1	0	5	0	3
45	4	max	242.418	2	1.205	6	.015	10	0	5	0	1	0	15
46		min	-224.881	3	.283	15	-.904	4	0	1	0	5	0	4
47	5	max	242.348	2	1.029	6	.015	10	0	5	0	1	0	15
48		min	-224.934	3	.241	15	-.771	4	0	1	0	5	0	4
49	6	max	242.278	2	.852	6	.015	10	0	5	0	1	0	15
50		min	-224.986	3	.2	15	-.637	4	0	1	0	5	0	4
51	7	max	242.208	2	.676	6	.015	10	0	5	0	1	0	15
52		min	-225.039	3	.158	15	-.503	4	0	1	0	5	0	4
53	8	max	242.138	2	.5	6	.015	10	0	5	0	1	0	15
54		min	-225.091	3	.117	15	-.37	4	0	1	0	5	-.001	4
55	9	max	242.068	2	.323	6	.015	10	0	5	0	1	0	15
56		min	-225.144	3	.075	15	-.236	4	0	1	0	5	-.001	4
57	10	max	241.998	2	.147	6	.015	10	0	5	0	1	0	15
58		min	-225.196	3	.034	15	-.102	4	0	1	0	5	-.001	4
59	11	max	241.928	2	.007	2	.053	5	0	5	0	1	0	15
60		min	-225.249	3	-.054	3	-.072	1	0	1	0	5	-.001	4
61	12	max	241.858	2	-.049	15	.187	5	0	5	0	1	0	15
62		min	-225.301	3	-.206	4	-.072	1	0	1	0	5	-.001	4
63	13	max	241.788	2	-.091	15	.32	5	0	5	0	1	0	15
64		min	-225.354	3	-.382	4	-.072	1	0	1	0	5	-.001	4
65	14	max	241.718	2	-.132	15	.454	5	0	5	0	1	0	15
66		min	-225.406	3	-.559	4	-.072	1	0	1	0	5	-.001	4
67	15	max	241.648	2	-.174	15	.588	5	0	5	0	9	0	15
68		min	-225.459	3	-.735	4	-.072	1	0	1	0	5	0	4
69	16	max	241.578	2	-.215	15	.721	5	0	5	0	9	0	15
70		min	-225.511	3	-.911	4	-.072	1	0	1	0	5	0	4
71	17	max	241.508	2	-.256	15	.855	5	0	5	0	10	0	15
72		min	-225.564	3	-1.088	4	-.072	1	0	1	0	4	0	4
73	18	max	241.438	2	-.298	15	.989	5	0	5	0	10	0	15
74		min	-225.616	3	-1.264	4	-.072	1	0	1	0	4	0	4
75	19	max	241.368	2	-.339	15	1.122	5	0	5	0	5	0	1
76		min	-225.669	3	-1.44	4	-.072	1	0	1	0	1	0	1
77	M4	1	max	204.28	1	0	.081	10	0	1	0	5	0	1
78		min	20.317	15	0	1	-12.556	4	0	1	0	2	0	1
79	2	max	204.345	1	0	1	.081	10	0	1	0	10	0	1
80		min	20.337	15	0	1	-12.612	4	0	1	-.001	4	0	1
81	3	max	204.409	1	0	1	.081	10	0	1	0	10	0	1
82		min	20.356	15	0	1	-12.668	4	0	1	-.002	4	0	1
83	4	max	204.474	1	0	1	.081	10	0	1	0	10	0	1
84		min	20.376	15	0	1	-12.724	4	0	1	-.003	4	0	1
85	5	max	204.539	1	0	1	.081	10	0	1	0	10	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	20.395	15	0	1	-12.78	4	0	1	-.005	4	0	1
87		6	max	204.603	1	0	1	.081	10	0	1	0	10	0	1
88			min	20.415	15	0	1	-12.836	4	0	1	-.006	4	0	1
89		7	max	204.668	1	0	1	.081	10	0	1	0	10	0	1
90			min	20.434	15	0	1	-12.892	4	0	1	-.007	4	0	1
91		8	max	204.733	1	0	1	.081	10	0	1	0	10	0	1
92			min	20.454	15	0	1	-12.948	4	0	1	-.008	4	0	1
93		9	max	204.797	1	0	1	.081	10	0	1	0	10	0	1
94			min	20.473	15	0	1	-13.004	4	0	1	-.009	4	0	1
95		10	max	204.862	1	0	1	.081	10	0	1	0	10	0	1
96			min	20.493	15	0	1	-13.06	4	0	1	-.01	4	0	1
97		11	max	204.927	1	0	1	.081	10	0	1	0	10	0	1
98			min	20.512	15	0	1	-13.117	4	0	1	-.011	4	0	1
99		12	max	204.992	1	0	1	.081	10	0	1	0	10	0	1
100			min	20.532	15	0	1	-13.173	4	0	1	-.013	4	0	1
101		13	max	205.056	1	0	1	.081	10	0	1	0	10	0	1
102			min	20.551	15	0	1	-13.229	4	0	1	-.014	4	0	1
103		14	max	205.121	1	0	1	.081	10	0	1	0	10	0	1
104			min	20.571	15	0	1	-13.285	4	0	1	-.015	4	0	1
105		15	max	205.186	1	0	1	.081	10	0	1	0	10	0	1
106			min	20.59	15	0	1	-13.341	4	0	1	-.016	4	0	1
107		16	max	205.25	1	0	1	.081	10	0	1	0	10	0	1
108			min	20.61	15	0	1	-13.397	4	0	1	-.017	4	0	1
109		17	max	205.315	1	0	1	.081	10	0	1	0	10	0	1
110			min	20.629	15	0	1	-13.453	4	0	1	-.019	4	0	1
111		18	max	205.38	1	0	1	.081	10	0	1	0	10	0	1
112			min	20.649	15	0	1	-13.509	4	0	1	-.02	4	0	1
113		19	max	205.445	1	0	1	.081	10	0	1	0	10	0	1
114			min	20.669	15	0	1	-13.565	4	0	1	-.021	4	0	1
115	M6	1	max	555.046	2	.656	6	1.026	4	0	3	0	3	0	1
116			min	-1008.426	3	.144	15	-.309	3	0	5	0	1	0	1
117		2	max	555.181	2	.599	6	.902	4	0	3	0	3	0	15
118			min	-1008.325	3	.13	15	-.309	3	0	5	0	1	0	6
119		3	max	555.316	2	.541	6	.779	4	0	3	0	4	0	15
120			min	-1008.224	3	.117	15	-.309	3	0	5	0	1	0	6
121		4	max	555.45	2	.484	6	.656	4	0	3	0	4	0	15
122			min	-1008.123	3	.103	15	-.309	3	0	5	0	1	0	6
123		5	max	555.585	2	.436	2	.533	4	0	3	0	4	0	15
124			min	-1008.022	3	.09	15	-.309	3	0	5	0	1	0	6
125		6	max	555.72	2	.391	2	.41	4	0	3	0	4	0	15
126			min	-1007.92	3	.076	15	-.309	3	0	5	0	1	0	6
127		7	max	555.855	2	.346	2	.287	4	0	3	0	4	0	15
128			min	-1007.819	3	.059	12	-.309	3	0	5	0	1	0	6
129		8	max	555.99	2	.301	2	.163	4	0	3	0	4	0	15
130			min	-1007.718	3	.037	12	-.309	3	0	5	0	3	0	2
131		9	max	556.125	2	.256	2	.04	4	0	3	0	4	0	15
132			min	-1007.617	3	.01	3	-.309	3	0	5	0	3	0	2
133		10	max	556.26	2	.212	2	.005	9	0	3	0	4	0	15
134			min	-1007.516	3	-.024	3	-.309	3	0	5	0	3	0	2
135		11	max	556.395	2	.167	2	.005	9	0	3	0	4	0	15
136			min	-1007.415	3	-.057	3	-.309	3	0	5	0	3	0	2
137		12	max	556.529	2	.122	2	.005	9	0	3	0	4	0	15
138			min	-1007.313	3	-.091	3	-.332	5	0	5	0	3	0	2
139		13	max	556.664	2	.077	2	.005	9	0	3	0	4	0	12
140			min	-1007.212	3	-.124	3	-.455	5	0	5	0	3	0	2
141		14	max	556.799	2	.032	2	.005	9	0	3	0	4	0	12
142			min	-1007.111	3	-.158	3	-.578	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	556.934	2	-.012	2	.005	9	0	3	0	4	0	12
144		min	-1007.01	3	-.192	3	-.701	5	0	5	0	3	0	2
145	16	max	557.069	2	-.057	2	.005	9	0	3	0	4	0	12
146		min	-1006.909	3	-.225	3	-.825	5	0	5	0	3	0	2
147	17	max	557.204	2	-.073	15	.005	9	0	3	0	4	0	3
148		min	-1006.808	3	-.265	4	-.948	5	0	5	0	3	0	2
149	18	max	557.339	2	-.086	15	.005	9	0	3	0	9	0	3
150		min	-1006.707	3	-.322	4	-1.071	5	0	5	0	3	0	2
151	19	max	557.473	2	-.1	15	.005	9	0	3	0	9	0	3
152		min	-1006.605	3	-.379	4	-1.194	5	0	5	0	3	0	2
153	M7	1	max	690.085	2	1.76	.058	3	0	9	0	4	0	2
154		min	-582.902	3	.423	15	-1.293	4	0	3	0	3	0	3
155	2	max	690.015	2	1.583	4	.058	3	0	9	0	4	0	2
156		min	-582.954	3	.382	15	-1.159	4	0	3	0	3	0	3
157	3	max	689.945	2	1.407	4	.058	3	0	9	0	9	0	2
158		min	-583.007	3	.34	15	-1.026	4	0	3	0	3	0	3
159	4	max	689.875	2	1.231	4	.058	3	0	9	0	9	0	2
160		min	-583.059	3	.299	15	-.892	4	0	3	0	3	0	3
161	5	max	689.805	2	1.054	4	.058	3	0	9	0	9	0	15
162		min	-583.112	3	.257	15	-.758	4	0	3	0	5	0	3
163	6	max	689.735	2	.878	4	.058	3	0	9	0	9	0	15
164		min	-583.164	3	.216	15	-.625	4	0	3	0	5	0	6
165	7	max	689.665	2	.702	4	.058	3	0	9	0	9	0	15
166		min	-583.217	3	.174	15	-.491	4	0	3	0	5	0	6
167	8	max	689.595	2	.525	4	.058	3	0	9	0	9	0	15
168		min	-583.269	3	.133	15	-.357	4	0	3	0	5	-.001	6
169	9	max	689.525	2	.349	4	.058	3	0	9	0	9	0	15
170		min	-583.322	3	.07	12	-.224	4	0	3	-.001	5	-.001	6
171	10	max	689.455	2	.195	2	.058	3	0	9	0	9	0	15
172		min	-583.374	3	-.008	3	-.09	4	0	3	-.001	5	-.001	6
173	11	max	689.385	2	.058	2	.058	3	0	9	0	9	0	15
174		min	-583.427	3	-.111	3	-.002	9	0	3	-.001	5	-.001	6
175	12	max	689.315	2	-.033	15	.178	5	0	9	0	9	0	15
176		min	-583.479	3	-.214	3	-.002	9	0	3	-.001	5	-.001	6
177	13	max	689.245	2	-.074	15	.312	5	0	9	0	9	0	15
178		min	-583.532	3	-.357	6	-.002	9	0	3	0	5	-.001	6
179	14	max	689.175	2	-.116	15	.446	5	0	9	0	9	0	15
180		min	-583.584	3	-.534	6	-.002	9	0	3	0	5	-.001	6
181	15	max	689.105	2	-.157	15	.579	5	0	9	0	9	0	15
182		min	-583.637	3	-.71	6	-.002	9	0	3	0	5	0	6
183	16	max	689.035	2	-.199	15	.713	5	0	9	0	9	0	15
184		min	-583.689	3	-.887	6	-.002	9	0	3	0	5	0	6
185	17	max	688.965	2	-.24	15	.847	5	0	9	0	9	0	15
186		min	-583.742	3	-1.063	6	-.002	9	0	3	0	5	0	6
187	18	max	688.895	2	-.282	15	.98	5	0	9	0	9	0	15
188		min	-583.794	3	-1.239	6	-.002	9	0	3	0	3	0	6
189	19	max	688.825	2	-.323	15	1.114	5	0	9	0	9	0	1
190		min	-583.847	3	-1.416	6	-.002	9	0	3	0	3	0	1
191	M8	1	max	572.567	1	0	.057	9	0	1	0	4	0	1
192		min	14.904	15	0	1	-12.799	4	0	1	0	3	0	1
193	2	max	572.632	1	0	1	.057	9	0	1	0	9	0	1
194		min	14.924	15	0	1	-12.855	4	0	1	-.001	4	0	1
195	3	max	572.697	1	0	1	.057	9	0	1	0	9	0	1
196		min	14.943	15	0	1	-12.911	4	0	1	-.002	4	0	1
197	4	max	572.762	1	0	1	.057	9	0	1	0	9	0	1
198		min	14.963	15	0	1	-12.967	4	0	1	-.003	4	0	1
199	5	max	572.826	1	0	1	.057	9	0	1	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	14.982	15	0	1	-13.023	4	0	1	-.005	4	0	1
201		6	max	572.891	1	0	1	.057	9	0	1	0	9	0	1
202			min	15.002	15	0	1	-13.079	4	0	1	-.006	4	0	1
203		7	max	572.956	1	0	1	.057	9	0	1	0	9	0	1
204			min	15.021	15	0	1	-13.135	4	0	1	-.007	4	0	1
205		8	max	573.02	1	0	1	.057	9	0	1	0	9	0	1
206			min	15.041	15	0	1	-13.191	4	0	1	-.008	4	0	1
207		9	max	573.085	1	0	1	.057	9	0	1	0	9	0	1
208			min	15.06	15	0	1	-13.247	4	0	1	-.009	4	0	1
209		10	max	573.15	1	0	1	.057	9	0	1	0	9	0	1
210			min	15.08	15	0	1	-13.303	4	0	1	-.01	4	0	1
211		11	max	573.215	1	0	1	.057	9	0	1	0	9	0	1
212			min	15.099	15	0	1	-13.359	4	0	1	-.012	4	0	1
213		12	max	573.279	1	0	1	.057	9	0	1	0	9	0	1
214			min	15.119	15	0	1	-13.416	4	0	1	-.013	4	0	1
215		13	max	573.344	1	0	1	.057	9	0	1	0	9	0	1
216			min	15.138	15	0	1	-13.472	4	0	1	-.014	4	0	1
217		14	max	573.409	1	0	1	.057	9	0	1	0	9	0	1
218			min	15.158	15	0	1	-13.528	4	0	1	-.015	4	0	1
219		15	max	573.473	1	0	1	.057	9	0	1	0	9	0	1
220			min	15.177	15	0	1	-13.584	4	0	1	-.017	4	0	1
221		16	max	573.538	1	0	1	.057	9	0	1	0	9	0	1
222			min	15.197	15	0	1	-13.64	4	0	1	-.018	4	0	1
223		17	max	573.603	1	0	1	.057	9	0	1	0	9	0	1
224			min	15.216	15	0	1	-13.696	4	0	1	-.019	4	0	1
225		18	max	573.667	1	0	1	.057	9	0	1	0	9	0	1
226			min	15.236	15	0	1	-13.752	4	0	1	-.02	4	0	1
227		19	max	573.732	1	0	1	.057	9	0	1	0	9	0	1
228			min	15.255	15	0	1	-13.808	4	0	1	-.021	4	0	1
229	M10	1	max	187.236	2	.714	4	1.116	5	0	1	0	9	0	1
230			min	-245.321	3	.183	15	-.047	1	0	5	0	3	0	1
231		2	max	187.371	2	.657	4	.992	5	0	1	0	4	0	15
232			min	-245.22	3	.17	15	-.047	1	0	5	0	3	0	4
233		3	max	187.506	2	.599	4	.869	5	0	1	0	4	0	15
234			min	-245.119	3	.156	15	-.047	1	0	5	0	3	0	4
235		4	max	187.641	2	.542	4	.746	5	0	1	0	4	0	15
236			min	-245.018	3	.143	15	-.047	1	0	5	0	3	0	4
237		5	max	187.776	2	.484	4	.623	5	0	1	0	4	0	15
238			min	-244.916	3	.129	15	-.047	1	0	5	0	3	0	4
239		6	max	187.91	2	.427	4	.5	5	0	1	0	4	0	15
240			min	-244.815	3	.116	15	-.047	1	0	5	0	3	0	4
241		7	max	188.045	2	.369	4	.377	5	0	1	0	4	0	15
242			min	-244.714	3	.102	15	-.047	1	0	5	0	3	0	4
243		8	max	188.18	2	.312	4	.253	5	0	1	0	4	0	15
244			min	-244.613	3	.085	12	-.047	1	0	5	0	3	0	4
245		9	max	188.315	2	.254	4	.13	5	0	1	0	5	0	15
246			min	-244.512	3	.063	12	-.047	1	0	5	0	3	0	4
247		10	max	188.45	2	.197	4	.008	10	0	1	0	5	0	15
248			min	-244.411	3	.04	12	-.047	1	0	5	0	3	0	4
249		11	max	188.585	2	.139	4	.008	10	0	1	0	5	0	15
250			min	-244.31	3	.018	12	-.128	4	0	5	0	3	0	4
251		12	max	188.72	2	.082	4	.008	10	0	1	0	5	0	15
252			min	-244.208	3	-.01	3	-.252	4	0	5	0	3	0	4
253		13	max	188.854	2	.031	5	.008	10	0	1	0	5	0	15
254			min	-244.107	3	-.044	3	-.375	4	0	5	0	3	0	4
255		14	max	188.989	2	.01	5	.008	10	0	1	0	5	0	15
256			min	-244.006	3	-.077	3	-.498	4	0	5	0	3	0	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	189.124	2	-.006	15	.008	10	0	1	0	5	0	12
258		min	-243.905	3	-.111	3	-.621	4	0	5	0	3	0	4
259	16	max	189.259	2	-.019	15	.008	10	0	1	0	5	0	12
260		min	-243.804	3	-.15	6	-.744	4	0	5	0	3	0	4
261	17	max	189.394	2	-.033	15	.008	10	0	1	0	5	0	12
262		min	-243.703	3	-.207	6	-.867	4	0	5	0	3	0	4
263	18	max	189.529	2	-.046	15	.008	10	0	1	0	5	0	12
264		min	-243.602	3	-.265	6	-.99	4	0	5	0	3	0	4
265	19	max	189.664	2	-.06	15	.008	10	0	1	0	10	0	12
266		min	-243.5	3	-.322	6	-1.114	4	0	5	0	3	0	4
267	M11	1	max	242.227	2	1.719	.072	1	0	4	0	3	0	2
268		min	-225.91	3	.396	15	-1.268	5	0	10	0	1	0	15
269	2	max	242.157	2	1.543	6	.072	1	0	4	0	3	0	2
270		min	-225.963	3	.355	15	-1.134	5	0	10	0	1	0	15
271	3	max	242.087	2	1.366	6	.072	1	0	4	0	3	0	2
272		min	-226.015	3	.313	15	-1	5	0	10	0	1	0	3
273	4	max	242.017	2	1.19	6	.072	1	0	4	0	3	0	15
274		min	-226.068	3	.272	15	-.867	5	0	10	0	4	0	4
275	5	max	241.947	2	1.014	6	.072	1	0	4	0	3	0	15
276		min	-226.12	3	.23	15	-.733	5	0	10	0	4	0	4
277	6	max	241.877	2	.837	6	.072	1	0	4	0	3	0	15
278		min	-226.173	3	.189	15	-.599	5	0	10	0	4	0	4
279	7	max	241.807	2	.661	6	.072	1	0	4	0	3	0	15
280		min	-226.225	3	.147	15	-.466	5	0	10	0	4	-.001	4
281	8	max	241.737	2	.484	6	.072	1	0	4	0	3	0	15
282		min	-226.278	3	.106	15	-.332	5	0	10	0	4	-.001	4
283	9	max	241.667	2	.308	6	.072	1	0	4	0	3	0	15
284		min	-226.33	3	.064	15	-.198	5	0	10	0	4	-.001	4
285	10	max	241.597	2	.144	2	.072	1	0	4	0	3	0	15
286		min	-226.383	3	.023	15	-.072	3	0	10	0	4	-.001	4
287	11	max	241.527	2	.007	2	.091	4	0	4	0	3	0	15
288		min	-226.435	3	-.048	3	-.072	3	0	10	0	4	-.001	4
289	12	max	241.457	2	-.06	15	.224	4	0	4	0	3	0	15
290		min	-226.488	3	-.222	4	-.072	3	0	10	0	4	-.001	4
291	13	max	241.387	2	-.101	15	.358	4	0	4	0	3	0	15
292		min	-226.54	3	-.398	4	-.072	3	0	10	0	4	-.001	4
293	14	max	241.317	2	-.143	15	.492	4	0	4	0	3	0	15
294		min	-226.593	3	-.575	4	-.072	3	0	10	0	4	-.001	4
295	15	max	241.247	2	-.184	15	.625	4	0	4	0	3	0	15
296		min	-226.645	3	-.751	4	-.072	3	0	10	0	4	0	4
297	16	max	241.177	2	-.226	15	.759	4	0	4	0	3	0	15
298		min	-226.698	3	-.927	4	-.072	3	0	10	0	5	0	4
299	17	max	241.107	2	-.267	15	.893	4	0	4	0	3	0	15
300		min	-226.75	3	-1.104	4	-.072	3	0	10	0	5	0	4
301	18	max	241.037	2	-.309	15	1.026	4	0	4	0	3	0	15
302		min	-226.803	3	-1.28	4	-.072	3	0	10	0	10	0	4
303	19	max	240.967	2	-.35	15	1.16	4	0	4	0	4	0	1
304		min	-226.855	3	-1.456	4	-.072	3	0	10	0	10	0	1
305	M12	1	max	204.68	1	0	.439	3	0	1	0	4	0	1
306		min	-1.548	5	0	1	-11.816	5	0	1	0	3	0	1
307	2	max	204.745	1	0	1	.439	3	0	1	0	1	0	1
308		min	-1.517	5	0	1	-11.872	5	0	1	-.001	5	0	1
309	3	max	204.809	1	0	1	.439	3	0	1	0	3	0	1
310		min	-1.487	5	0	1	-11.928	5	0	1	-.002	5	0	1
311	4	max	204.874	1	0	1	.439	3	0	1	0	3	0	1
312		min	-1.457	5	0	1	-11.985	5	0	1	-.003	5	0	1
313	5	max	204.939	1	0	1	.439	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-1.427	5	0	1	-12.041	5	0	1	-.004	5	0	1
315		6	max	205.004	1	0	1	.439	3	0	1	0	3	0	1
316			min	-1.397	5	0	1	-12.097	5	0	1	-.005	5	0	1
317		7	max	205.068	1	0	1	.439	3	0	1	0	3	0	1
318			min	-1.366	5	0	1	-12.153	5	0	1	-.006	5	0	1
319		8	max	205.133	1	0	1	.439	3	0	1	0	3	0	1
320			min	-1.336	5	0	1	-12.209	5	0	1	-.008	5	0	1
321		9	max	205.198	1	0	1	.439	3	0	1	0	3	0	1
322			min	-1.306	5	0	1	-12.265	5	0	1	-.009	5	0	1
323		10	max	205.262	1	0	1	.439	3	0	1	0	3	0	1
324			min	-1.276	5	0	1	-12.321	5	0	1	-.01	5	0	1
325		11	max	205.327	1	0	1	.439	3	0	1	0	3	0	1
326			min	-1.246	5	0	1	-12.377	5	0	1	-.011	5	0	1
327		12	max	205.392	1	0	1	.439	3	0	1	0	3	0	1
328			min	-1.216	5	0	1	-12.433	5	0	1	-.012	5	0	1
329		13	max	205.457	1	0	1	.439	3	0	1	0	3	0	1
330			min	-1.185	5	0	1	-12.489	5	0	1	-.013	5	0	1
331		14	max	205.521	1	0	1	.439	3	0	1	0	3	0	1
332			min	-1.155	5	0	1	-12.545	5	0	1	-.014	5	0	1
333		15	max	205.586	1	0	1	.439	3	0	1	0	3	0	1
334			min	-1.125	5	0	1	-12.601	5	0	1	-.015	5	0	1
335		16	max	205.651	1	0	1	.439	3	0	1	0	3	0	1
336			min	-1.095	5	0	1	-12.658	5	0	1	-.016	5	0	1
337		17	max	205.715	1	0	1	.439	3	0	1	0	3	0	1
338			min	-1.065	5	0	1	-12.714	5	0	1	-.018	5	0	1
339		18	max	205.78	1	0	1	.439	3	0	1	0	3	0	1
340			min	-1.037	15	0	1	-12.77	5	0	1	-.019	5	0	1
341		19	max	205.845	1	0	1	.439	3	0	1	0	3	0	1
342			min	-1.017	15	0	1	-12.826	5	0	1	-.02	5	0	1
343	M1	1	max	60.126	1	339.475	3	2.009	10	0	2	.025	4	0	2
344			min	3.152	10	-208.707	2	-14.369	4	0	3	-.004	10	0	3
345		2	max	60.287	1	339.304	3	2.009	10	0	2	.022	4	.046	2
346			min	3.285	10	-208.936	2	-14.127	4	0	3	-.004	10	-.074	3
347		3	max	120.144	3	4.364	4	2.001	10	0	10	.018	4	.09	2
348			min	-33.879	2	-30.634	2	-12.797	4	0	1	-.003	10	-.146	3
349		4	max	120.264	3	4.071	4	2.001	10	0	10	.015	4	.097	2
350			min	-33.719	2	-30.863	2	-12.555	4	0	1	-.003	10	-.145	3
351		5	max	120.384	3	3.777	4	2.001	10	0	10	.013	4	.104	2
352			min	-33.559	2	-31.091	2	-12.313	4	0	1	-.002	10	-.143	3
353		6	max	120.504	3	3.484	4	2.001	10	0	10	.01	4	.11	2
354			min	-33.399	2	-31.32	2	-12.071	4	0	1	-.002	10	-.141	3
355		7	max	120.624	3	3.19	4	2.001	10	0	10	.008	4	.117	2
356			min	-33.239	2	-31.549	2	-11.829	4	0	1	-.001	10	-.14	3
357		8	max	120.744	3	2.897	4	2.001	10	0	10	.005	3	.124	2
358			min	-33.078	2	-31.777	2	-11.587	4	0	1	0	10	-.138	3
359		9	max	120.864	3	2.659	14	2.001	10	0	10	.004	3	.131	2
360			min	-32.918	2	-32.006	2	-11.345	4	0	1	0	10	-.136	3
361		10	max	120.984	3	2.434	14	2.001	10	0	10	.002	3	.138	2
362			min	-32.758	2	-32.235	2	-11.103	4	0	1	0	2	-.134	3
363		11	max	121.105	3	2.21	14	2.001	10	0	10	0	3	.145	2
364			min	-32.598	2	-32.464	2	-10.861	4	0	1	-.002	4	-.132	3
365		12	max	121.225	3	1.985	14	2.001	10	0	10	0	10	.152	2
366			min	-32.438	2	-32.692	2	-10.619	4	0	1	-.005	4	-.13	3
367		13	max	121.345	3	1.76	14	2.001	10	0	10	.001	10	.159	2
368			min	-32.278	2	-32.921	2	-10.377	4	0	1	-.007	4	-.128	3
369		14	max	121.465	3	1.535	14	2.001	10	0	10	.002	10	.166	2
370			min	-32.117	2	-33.15	2	-10.159	14	0	1	-.009	4	-.126	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	121.585	3	1.311	14	2.001	10	0	10	.002	10	.174	2
372		min	-31.957	2	-33.379	2	-10.037	14	0	1	-.011	4	-.124	3
373	16	max	82.473	2	179.29	2	2.014	10	0	1	.003	10	.179	2
374		min	2.557	15	-209.264	3	-9.731	1	0	5	-.013	4	-.121	3
375	17	max	82.633	2	179.061	2	2.014	10	0	1	.003	10	.14	2
376		min	2.605	15	-209.436	3	-9.731	1	0	5	-.015	14	-.075	3
377	18	max	-3.311	10	323.287	2	2.095	10	0	5	.003	10	.071	2
378		min	-60.284	1	-172.366	3	-20.469	4	0	2	-.02	4	-.038	3
379	19	max	-3.177	10	323.058	2	2.095	10	0	5	.004	10	0	2
380		min	-60.124	1	-172.538	3	-20.227	4	0	2	-.024	4	0	3
381	M5	1	max	162.09	1	1043.292	3	0	1	0	.025	4	0	3
382		min	-12.624	3	-625.274	2	-96.842	3	0	3	0	11	0	2
383	2	max	162.25	1	1043.121	3	0	1	0	9	.022	4	.135	2
384		min	-12.504	3	-625.503	2	-96.842	3	0	3	-.006	3	-.226	3
385	3	max	309.829	3	4.209	9	10.235	3	0	3	.018	4	.269	2
386		min	-77.412	2	-94.084	2	-14.167	4	0	4	-.026	3	-.447	3
387	4	max	309.949	3	4.018	9	10.235	3	0	3	.015	4	.289	2
388		min	-77.252	2	-94.313	2	-13.925	4	0	4	-.023	3	-.439	3
389	5	max	310.069	3	3.828	9	10.235	3	0	3	.012	4	.309	2
390		min	-77.091	2	-94.541	2	-13.683	4	0	4	-.021	3	-.431	3
391	6	max	310.189	3	3.637	9	10.235	3	0	3	.009	4	.33	2
392		min	-76.931	2	-94.77	2	-13.441	4	0	4	-.019	3	-.423	3
393	7	max	310.31	3	3.446	9	10.235	3	0	3	.006	4	.351	2
394		min	-76.771	2	-94.999	2	-13.199	4	0	4	-.017	3	-.415	3
395	8	max	310.43	3	3.256	9	10.235	3	0	3	.003	4	.371	2
396		min	-76.611	2	-95.227	2	-12.957	4	0	4	-.015	3	-.407	3
397	9	max	310.55	3	3.065	9	10.235	3	0	3	0	4	.392	2
398		min	-76.451	2	-95.456	2	-12.715	4	0	4	-.012	3	-.399	3
399	10	max	310.67	3	2.875	9	10.235	3	0	3	0	1	.413	2
400		min	-76.291	2	-95.685	2	-12.473	4	0	4	-.01	3	-.391	3
401	11	max	310.79	3	2.684	9	10.235	3	0	3	0	1	.433	2
402		min	-76.13	2	-95.914	2	-12.231	4	0	4	-.008	3	-.383	3
403	12	max	310.91	3	2.493	9	10.235	3	0	3	0	1	.454	2
404		min	-75.97	2	-96.142	2	-11.989	4	0	4	-.008	4	-.375	3
405	13	max	311.03	3	2.303	9	10.235	3	0	3	0	1	.475	2
406		min	-75.81	2	-96.371	2	-11.747	4	0	4	-.01	4	-.367	3
407	14	max	311.15	3	2.112	9	10.235	3	0	3	0	1	.496	2
408		min	-75.65	2	-96.6	2	-11.505	4	0	4	-.013	4	-.359	3
409	15	max	311.271	3	1.921	9	10.235	3	0	3	0	3	.517	2
410		min	-75.49	2	-96.829	2	-11.263	4	0	4	-.015	4	-.351	3
411	16	max	248.701	2	513.413	2	10.222	3	0	3	.003	3	.533	2
412		min	-1.032	5	-554.477	3	-9.913	4	0	4	-.018	4	-.338	3
413	17	max	248.861	2	513.184	2	10.222	3	0	3	.005	3	.421	2
414		min	-.957	5	-554.648	3	-9.671	4	0	4	-.02	4	-.218	3
415	18	max	1.725	3	977.61	2	9.355	3	0	4	.007	3	.211	2
416		min	-162.254	1	-502.539	3	-22.03	5	0	9	-.025	4	-.108	3
417	19	max	1.845	3	977.381	2	9.355	3	0	4	.009	3	0	3
418		min	-162.094	1	-502.71	3	-21.788	5	0	9	-.029	4	0	2
419	M9	1	max	60.126	1	339.338	3	103.15	3	0	.004	10	0	2
420		min	.986	15	-208.707	2	-2.008	10	0	2	-.027	3	0	3
421	2	max	60.286	1	339.166	3	103.15	3	0	3	.019	5	.046	2
422		min	1.034	15	-208.936	2	-2.008	10	0	2	-.017	1	-.074	3
423	3	max	119.267	3	3.486	9	9.668	1	0	1	.038	5	.09	2
424		min	-33.461	2	-30.607	2	-18.041	5	0	5	-.015	1	-.146	3
425	4	max	119.387	3	3.296	9	9.668	1	0	1	.034	5	.097	2
426		min	-33.301	2	-30.836	2	-17.799	5	0	5	-.013	1	-.144	3
427	5	max	119.507	3	3.105	9	9.668	1	0	1	.03	5	.104	2



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-33.141	2	-31.065	2	-17.557	5	0	5	-.011	1	-.143	3
429	6	max	119.627	3	2.914	9	9.668	1	0	1	.026	5	.11	2
430		min	-32.981	2	-31.293	2	-17.315	5	0	5	-.008	1	-.141	3
431	7	max	119.747	3	2.724	9	9.668	1	0	1	.022	5	.117	2
432		min	-32.821	2	-31.522	2	-17.073	5	0	5	-.006	1	-.139	3
433	8	max	119.867	3	2.533	9	9.668	1	0	1	.019	5	.124	2
434		min	-32.66	2	-31.751	2	-16.831	5	0	5	-.004	1	-.138	3
435	9	max	119.987	3	2.343	9	9.668	1	0	1	.015	5	.131	2
436		min	-32.5	2	-31.98	2	-16.589	5	0	5	-.002	1	-.136	3
437	10	max	120.108	3	2.152	9	9.668	1	0	1	.013	3	.138	2
438		min	-32.34	2	-32.208	2	-16.347	5	0	5	0	1	-.134	3
439	11	max	120.228	3	1.961	9	9.668	1	0	1	.012	3	.145	2
440		min	-32.18	2	-32.437	2	-16.105	5	0	5	0	10	-.132	3
441	12	max	120.348	3	1.771	9	9.668	1	0	1	.012	3	.152	2
442		min	-32.02	2	-32.666	2	-15.863	5	0	5	0	10	-.13	3
443	13	max	120.468	3	1.58	9	9.668	1	0	1	.011	3	.159	2
444		min	-31.86	2	-32.895	2	-15.621	5	0	5	-.001	10	-.129	3
445	14	max	120.588	3	1.389	9	9.668	1	0	1	.011	3	.166	2
446		min	-31.699	2	-33.123	2	-15.379	5	0	5	-.002	5	-.127	3
447	15	max	120.708	3	1.199	9	9.668	1	0	1	.01	1	.173	2
448		min	-31.539	2	-33.352	2	-15.137	5	0	5	-.006	5	-.125	3
449	16	max	82.702	2	178.963	2	9.731	1	0	10	.013	1	.179	2
450		min	4.501	15	-210.148	3	-13.755	5	0	4	-.008	5	-.121	3
451	17	max	82.862	2	178.734	2	9.731	1	0	10	.015	1	.14	2
452		min	4.549	15	-210.319	3	-13.513	5	0	4	-.011	5	-.075	3
453	18	max	7.822	5	323.287	2	10.098	1	0	2	.017	1	.071	2
454		min	-60.284	1	-172.348	3	-25.09	5	0	3	-.017	5	-.038	3
455	19	max	7.897	5	323.058	2	10.098	1	0	2	.019	1	0	2
456		min	-60.124	1	-172.52	3	-24.848	5	0	3	-.022	5	0	3
457	M13	1	max	103.14	3	208.647	2	-.986	15	0	.027	3	0	2
458		min	-2.009	10	-339.42	3	-60.123	1	0	3	-.004	10	0	3
459	2	max	103.14	3	150.769	2	-.277	15	0	2	.021	3	.105	3
460		min	-2.009	10	-243.746	3	-44.438	1	0	3	-.007	2	-.065	2
461	3	max	103.14	3	92.891	2	.76	10	0	2	.017	3	.176	3
462		min	-2.009	10	-148.073	3	-28.752	1	0	3	-.013	1	-.109	2
463	4	max	103.14	3	35.013	2	2.716	10	0	2	.012	3	.212	3
464		min	-2.009	10	-52.399	3	-13.066	1	0	3	-.021	1	-.132	2
465	5	max	103.14	3	43.275	3	7.5	2	0	2	.008	3	.214	3
466		min	-2.009	10	-22.865	2	-10.917	3	0	3	-.022	1	-.134	2
467	6	max	103.14	3	138.949	3	18.305	1	0	2	.004	3	.181	3
468		min	-2.009	10	-80.742	2	-9.886	3	0	3	-.019	1	-.115	2
469	7	max	103.14	3	234.623	3	33.991	1	0	2	.005	5	.114	3
470		min	-2.009	10	-138.62	2	-8.856	3	0	3	-.009	1	-.076	2
471	8	max	103.14	3	330.296	3	49.677	1	0	2	.009	2	.012	3
472		min	-2.009	10	-196.498	2	-7.826	3	0	3	-.002	3	-.015	2
473	9	max	103.14	3	425.97	3	65.362	1	0	2	.027	1	.066	2
474		min	-2.009	10	-254.376	2	-6.795	3	0	3	-.005	3	-.125	3
475	10	max	103.14	3	-5.635	15	81.048	1	0	2	.053	1	.168	2
476		min	-2.009	10	-521.644	3	3.81	12	0	3	-.022	3	-.296	3
477	11	max	42.728	4	254.376	2	7.938	3	0	3	.027	1	.066	2
478		min	-2.009	10	-425.97	3	-65.362	1	0	2	-.02	3	-.125	3
479	12	max	39.181	4	196.498	2	8.969	3	0	3	.009	2	.012	3
480		min	-2.009	10	-330.296	3	-49.676	1	0	2	-.017	3	-.015	2
481	13	max	35.635	4	138.62	2	9.999	3	0	3	.002	10	.114	3
482		min	-2.009	10	-234.622	3	-33.991	1	0	2	-.013	3	-.076	2
483	14	max	32.088	4	80.742	2	11.029	3	0	3	0	10	.181	3
484		min	-2.009	10	-138.949	3	-18.305	1	0	2	-.019	1	-.115	2



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	28.541	4	22.865	2	12.06	3	0	3	0	5	.214	3
486			min	-2.009	10	-43.275	3	-7.5	2	0	2	-.022	1	-.134	2
487		16	max	24.994	4	52.399	3	15.576	4	0	3	.005	5	.212	3
488			min	-2.009	10	-35.013	2	-2.716	10	0	2	-.021	1	-.132	2
489		17	max	21.448	4	148.073	3	28.752	1	0	3	.009	5	.176	3
490			min	-2.009	10	-92.891	2	-.76	10	0	2	-.013	1	-.109	2
491		18	max	17.901	4	243.746	3	44.438	1	0	3	.015	4	.105	3
492			min	-2.009	10	-150.769	2	1.196	10	0	2	-.007	2	-.065	2
493		19	max	14.354	4	339.42	3	60.124	1	0	3	.025	4	0	2
494			min	-2.009	10	-208.647	2	3.152	10	0	2	-.004	10	0	3
495	M16	1	max	24.84	5	323.139	2	7.897	5	0	3	.019	1	0	2
496			min	-10.084	1	-172.56	3	-60.127	1	0	2	-.022	5	0	3
497		2	max	21.294	5	232.879	2	8.992	5	0	3	.004	3	.054	3
498			min	-10.084	1	-125.459	3	-44.442	1	0	2	-.019	5	-.1	2
499		3	max	17.747	5	142.62	2	10.088	5	0	3	0	3	.091	3
500			min	-10.084	1	-78.358	3	-28.756	1	0	2	-.018	4	-.168	2
501		4	max	14.2	5	52.36	2	11.183	5	0	3	-.001	12	.11	3
502			min	-10.084	1	-31.257	3	-13.07	1	0	2	-.021	1	-.203	2
503		5	max	10.654	5	15.844	3	12.278	5	0	3	-.003	10	.113	3
504			min	-10.084	1	-37.9	2	-7.11	3	0	2	-.022	1	-.206	2
505		6	max	7.107	5	62.945	3	18.301	1	0	3	0	10	.099	3
506			min	-10.084	1	-128.16	2	-6.079	3	0	2	-.019	1	-.176	2
507		7	max	3.56	5	110.046	3	33.987	1	0	3	.002	5	.068	3
508			min	-10.084	1	-218.419	2	-5.049	3	0	2	-.009	3	-.113	2
509		8	max	2.362	3	157.147	3	49.673	1	0	3	.009	2	.019	3
510			min	-10.084	1	-308.679	2	-4.018	3	0	2	-.011	3	-.018	2
511		9	max	2.362	3	204.248	3	65.359	1	0	3	.027	1	.109	2
512			min	-10.084	1	-398.939	2	-2.988	3	0	2	-.012	3	-.046	3
513		10	max	14.714	5	251.349	3	81.044	1	0	14	.053	1	.27	2
514			min	-10.28	14	-489.198	2	-1.958	3	0	2	-.013	3	-.128	3
515		11	max	11.167	5	398.939	2	4.717	5	0	2	.027	1	.109	2
516			min	-10.084	1	-204.248	3	-65.358	1	0	3	-.008	5	-.046	3
517		12	max	7.62	5	308.679	2	5.812	5	0	2	.009	2	.019	3
518			min	-10.084	1	-157.147	3	-49.673	1	0	3	-.006	5	-.018	2
519		13	max	4.073	5	218.419	2	6.908	5	0	2	.002	10	.068	3
520			min	-10.084	1	-110.046	3	-33.987	1	0	3	-.009	1	-.113	2
521		14	max	2.095	10	128.16	2	8.003	5	0	2	0	15	.099	3
522			min	-10.084	1	-62.945	3	-18.301	1	0	3	-.019	1	-.176	2
523		15	max	2.095	10	37.9	2	9.467	4	0	2	.002	5	.113	3
524			min	-10.084	1	-15.844	3	-7.464	2	0	3	-.022	1	-.206	2
525		16	max	2.095	10	31.257	3	14.147	4	0	2	.005	5	.11	3
526			min	-10.084	1	-52.36	2	-2.691	10	0	3	-.021	1	-.203	2
527		17	max	2.095	10	78.358	3	28.756	1	0	2	.009	5	.091	3
528			min	-13.15	4	-142.62	2	-.735	10	0	3	-.013	1	-.168	2
529		18	max	2.095	10	125.46	3	44.442	1	0	2	.015	4	.054	3
530			min	-16.696	4	-232.879	2	1.221	10	0	3	-.007	2	-.1	2
531		19	max	2.095	10	172.561	3	60.127	1	0	2	.024	4	0	2
532			min	-20.243	4	-323.139	2	3.177	10	0	3	-.004	10	0	5
533	M15	1	max	0	1	.696	3	.173	3	0	1	0	1	0	1
534			min	-158.227	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.618	3	.173	3	0	1	0	1	0	1
536			min	-158.303	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.541	3	.173	3	0	1	0	1	0	1
538			min	-158.378	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.464	3	.173	3	0	1	0	1	0	1
540			min	-158.454	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.387	3	.173	3	0	1	0	1	0	1



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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-158.529	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.309	3	.173	3	0	1	0	1	0	1
544		min	-158.605	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.232	3	.173	3	0	1	0	3	0	1
546		min	-158.68	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.155	3	.173	3	0	1	0	3	0	1
548		min	-158.756	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.077	3	.173	3	0	1	0	3	0	1
550		min	-158.831	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.173	3	0	1	0	3	0	1
552		min	-158.907	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.173	3	0	1	0	3	0	1
554		min	-158.982	3	-.077	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.173	3	0	1	0	3	0	1
556		min	-159.058	3	-.155	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.173	3	0	1	0	3	0	1
558		min	-159.133	3	-.232	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.173	3	0	1	0	3	0	1
560		min	-159.209	3	-.309	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.173	3	0	1	0	3	0	1
562		min	-159.284	3	-.387	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.173	3	0	1	0	3	0	1
564		min	-159.36	3	-.464	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.173	3	0	1	0	3	0	1
566		min	-159.435	3	-.541	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.173	3	0	1	0	3	0	1
568		min	-159.511	3	-.618	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.173	3	0	1	0	3	0	1
570		min	-159.587	3	-.696	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.934	.389	4	0	3	0	3	0	1
572		min	-167.33	4	0	1	-.073	3	0	4	0	4	0	1
573	2	max	0	1	1.719	4	.348	4	0	3	0	3	0	1
574		min	-167.285	4	0	1	-.073	3	0	4	0	4	0	4
575	3	max	0	1	1.504	4	.308	4	0	3	0	3	0	1
576		min	-167.24	4	0	1	-.073	3	0	4	0	4	0	4
577	4	max	0	1	1.289	4	.267	4	0	3	0	3	0	1
578		min	-167.196	4	0	1	-.073	3	0	4	0	4	-.001	4
579	5	max	0	1	1.074	4	.227	4	0	3	0	3	0	1
580		min	-167.151	4	0	1	-.073	3	0	4	0	9	-.002	4
581	6	max	0	1	.859	4	.186	4	0	3	0	3	0	1
582		min	-167.106	4	0	1	-.073	3	0	4	0	9	-.002	4
583	7	max	0	1	.645	4	.146	4	0	3	0	3	0	1
584		min	-167.061	4	0	1	-.073	3	0	4	0	9	-.002	4
585	8	max	0	1	.43	4	.105	4	0	3	0	5	0	1
586		min	-167.017	4	0	1	-.073	3	0	4	0	9	-.002	4
587	9	max	0	1	.215	4	.065	4	0	3	0	5	0	1
588		min	-166.972	4	0	1	-.073	3	0	4	0	9	-.002	4
589	10	max	0	1	0	1	.024	4	0	3	0	5	0	1
590		min	-166.927	4	0	1	-.073	3	0	4	0	9	-.002	4
591	11	max	0	1	0	1	.009	9	0	3	0	5	0	1
592		min	-166.882	4	-.215	4	-.073	3	0	4	0	9	-.002	4
593	12	max	0	1	0	1	.009	9	0	3	0	5	0	1
594		min	-166.838	4	-.43	4	-.073	3	0	4	0	9	-.002	4
595	13	max	0	1	0	1	.009	9	0	3	0	5	0	1
596		min	-166.793	4	-.645	4	-.099	5	0	4	0	3	-.002	4
597	14	max	.065	9	0	1	.009	9	0	3	0	5	0	1
598		min	-166.753	5	-.859	4	-.139	5	0	4	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.149	9	0	1	.009	9	0	3	0	5	0	1
600		min	-166.79	5	-1.074	4	-.18	5	0	4	0	3	-.002	4
601	16	max	.233	9	0	1	.009	9	0	3	0	5	0	1
602		min	-166.828	5	-1.289	4	-.22	5	0	4	0	3	-.001	4
603	17	max	.316	9	0	1	.009	9	0	3	0	9	0	1
604		min	-166.865	5	-1.504	4	-.261	5	0	4	0	3	0	4
605	18	max	.4	9	0	1	.009	9	0	3	0	9	0	1
606		min	-166.903	5	-1.719	4	-.301	5	0	4	0	4	0	4
607	19	max	.484	9	0	1	.009	9	0	3	0	9	0	1
608		min	-166.94	5	-1.934	4	-.342	5	0	4	0	4	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.011	2	.001	9	7.037e-4	5	NC	3	NC	1	
2			min	-.004	3	-.011	3	-.009	5	-2.483e-4	3	3911.628	2	NC	1	
3			2	max	.002	2	.01	2	.001	9	7.252e-4	5	NC	3	NC	1
4				min	-.004	3	-.011	3	-.009	5	-2.347e-4	3	4277.787	2	NC	1
5			3	max	.002	2	.009	2	.001	9	7.467e-4	5	NC	3	NC	1
6				min	-.003	3	-.01	3	-.009	5	-2.212e-4	3	4714.838	2	NC	1
7			4	max	.002	2	.008	2	.001	9	7.682e-4	5	NC	1	NC	1
8				min	-.003	3	-.01	3	-.009	5	-2.077e-4	3	5240.088	2	NC	1
9			5	max	.002	2	.007	2	0	9	7.896e-4	5	NC	1	NC	1
10				min	-.003	3	-.009	3	-.009	5	-1.941e-4	3	5876.657	2	NC	1
11			6	max	.001	2	.006	2	0	9	8.111e-4	5	NC	1	NC	1
12				min	-.003	3	-.009	3	-.008	5	-1.806e-4	3	6655.927	2	NC	1
13			7	max	.001	2	.006	2	0	9	8.326e-4	5	NC	1	NC	1
14				min	-.003	3	-.008	3	-.008	5	-1.671e-4	3	7621.306	2	NC	1
15			8	max	.001	2	.005	2	0	9	8.541e-4	5	NC	1	NC	1
16				min	-.002	3	-.008	3	-.008	5	-1.535e-4	3	8834.153	2	NC	1
17			9	max	.001	2	.004	2	0	9	8.756e-4	5	NC	1	NC	1
18				min	-.002	3	-.007	3	-.007	5	-1.4e-4	3	NC	1	NC	1
19			10	max	0	2	.003	2	0	9	8.971e-4	5	NC	1	NC	1
20				min	-.002	3	-.007	3	-.007	5	-1.264e-4	3	NC	1	NC	1
21		11	max	0	2	.003	2	0	9	9.186e-4	5	NC	1	NC	1	
22			min	-.002	3	-.006	3	-.006	5	-1.129e-4	3	NC	1	NC	1	
23		12	max	0	2	.002	2	0	9	9.401e-4	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.006	5	-9.937e-5	3	NC	1	NC	1	
25		13	max	0	2	.002	2	0	9	9.616e-4	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.005	5	-8.583e-5	3	NC	1	NC	1	
27		14	max	0	2	.001	2	0	9	9.831e-4	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.004	5	-7.229e-5	3	NC	1	NC	1	
29		15	max	0	2	0	2	0	9	1.005e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.003	5	-5.875e-5	3	NC	1	NC	1	
31		16	max	0	2	0	2	0	9	1.026e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.003	5	-4.522e-5	3	NC	1	NC	1	
33		17	max	0	2	0	2	0	9	1.048e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.002	5	-3.728e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	9	1.069e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-3.013e-5	9	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.091e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.379e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.142e-5	9	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.212e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.003	5	1.494e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	9	-5.224e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43	3	max	0	3	0	2	.005	5	1.931e-5	1	NC	1	NC	1
44		min	0	2	-.002	3	0	9	-5.237e-4	5	NC	1	NC	1
45	4	max	0	3	0	2	.008	5	2.368e-5	1	NC	1	NC	1
46		min	0	2	-.003	3	0	9	-5.249e-4	5	NC	1	NC	1
47	5	max	0	3	0	2	.011	4	2.806e-5	1	NC	1	NC	1
48		min	0	2	-.004	3	0	9	-5.261e-4	5	NC	1	NC	1
49	6	max	0	3	0	2	.013	4	3.243e-5	1	NC	1	NC	1
50		min	0	2	-.005	3	0	9	-5.274e-4	5	NC	1	NC	1
51	7	max	0	3	0	2	.016	4	3.68e-5	1	NC	1	NC	1
52		min	0	2	-.005	3	0	9	-5.286e-4	5	NC	1	NC	1
53	8	max	0	3	.001	2	.018	4	4.118e-5	1	NC	1	NC	1
54		min	-.001	2	-.006	3	0	10	-5.298e-4	5	NC	1	NC	1
55	9	max	.001	3	.001	2	.02	4	4.555e-5	1	NC	1	NC	1
56		min	-.001	2	-.007	3	0	10	-5.311e-4	5	NC	1	NC	1
57	10	max	.001	3	.002	2	.023	4	4.992e-5	1	NC	1	NC	1
58		min	-.001	2	-.007	3	0	10	-5.323e-4	5	NC	1	NC	1
59	11	max	.001	3	.002	2	.025	4	5.43e-5	1	NC	1	NC	1
60		min	-.002	2	-.008	3	0	10	-5.335e-4	5	NC	1	NC	1
61	12	max	.002	3	.003	2	.027	4	5.867e-5	1	NC	1	NC	1
62		min	-.002	2	-.008	3	0	10	-5.348e-4	5	NC	1	NC	1
63	13	max	.002	3	.004	2	.029	4	6.304e-5	1	NC	1	NC	1
64		min	-.002	2	-.008	3	0	10	-5.36e-4	5	NC	1	NC	1
65	14	max	.002	3	.005	2	.031	4	6.742e-5	1	NC	1	NC	1
66		min	-.002	2	-.009	3	0	10	-5.372e-4	5	NC	1	NC	1
67	15	max	.002	3	.005	2	.032	4	7.179e-5	1	NC	1	NC	1
68		min	-.002	2	-.009	3	0	10	-5.385e-4	5	8497.803	2	NC	1
69	16	max	.002	3	.006	2	.034	4	7.616e-5	1	NC	1	NC	1
70		min	-.002	2	-.009	3	0	10	-5.397e-4	5	7218.294	2	NC	1
71	17	max	.002	3	.007	2	.036	4	8.054e-5	1	NC	1	NC	1
72		min	-.002	2	-.009	3	0	10	-5.409e-4	5	6225.012	2	NC	1
73	18	max	.002	3	.008	2	.037	4	8.491e-5	1	NC	1	NC	1
74		min	-.003	2	-.009	3	0	10	-5.422e-4	5	5445.404	2	NC	1
75	19	max	.003	3	.01	2	.039	4	8.928e-5	1	NC	3	NC	1
76		min	-.003	2	-.009	3	0	10	-5.434e-4	5	4828.204	2	NC	1
77	M4	1	max	0	.012	2	0	10	3.016e-3	5	NC	1	NC	1
78		min	0	15	-.011	3	-.041	4	-1.097e-4	1	NC	1	473.644	4
79	2	max	0	1	.012	2	0	10	3.016e-3	5	NC	1	NC	1
80		min	0	15	-.011	3	-.037	4	-1.097e-4	1	NC	1	516.245	4
81	3	max	0	1	.011	2	0	10	3.016e-3	5	NC	1	NC	1
82		min	0	15	-.01	3	-.034	4	-1.097e-4	1	NC	1	566.936	4
83	4	max	0	1	.01	2	0	10	3.016e-3	5	NC	1	NC	1
84		min	0	15	-.009	3	-.031	4	-1.097e-4	1	NC	1	627.85	4
85	5	max	0	1	.01	2	0	10	3.016e-3	5	NC	1	NC	1
86		min	0	15	-.009	3	-.028	4	-1.097e-4	1	NC	1	701.888	4
87	6	max	0	1	.009	2	0	10	3.016e-3	5	NC	1	NC	1
88		min	0	15	-.008	3	-.024	4	-1.097e-4	1	NC	1	793.086	4
89	7	max	0	1	.008	2	0	10	3.016e-3	5	NC	1	NC	1
90		min	0	15	-.007	3	-.021	4	-1.097e-4	1	NC	1	907.191	4
91	8	max	0	1	.008	2	0	10	3.016e-3	5	NC	1	NC	1
92		min	0	15	-.007	3	-.018	4	-1.097e-4	1	NC	1	1052.619	4
93	9	max	0	1	.007	2	0	10	3.016e-3	5	NC	1	NC	1
94		min	0	15	-.006	3	-.016	4	-1.097e-4	1	NC	1	1242.095	4
95	10	max	0	1	.006	2	0	10	3.016e-3	5	NC	1	NC	1
96		min	0	15	-.006	3	-.013	4	-1.097e-4	1	NC	1	1495.622	4
97	11	max	0	1	.006	2	0	10	3.016e-3	5	NC	1	NC	1
98		min	0	15	-.005	3	-.01	4	-1.097e-4	1	NC	1	1846.144	4
99	12	max	0	1	.005	2	0	10	3.016e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.008	4	-1.097e-4	1	NC	1	2351.114	4
101		max	0	1	.004	2	0	10	3.016e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.006	4	-1.097e-4	1	NC	1	3118.297	4
103		max	0	1	.003	2	0	10	3.016e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.004	4	-1.097e-4	1	NC	1	4369.951	4
105		max	0	1	.003	2	0	10	3.016e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.003	4	-1.097e-4	1	NC	1	6628.191	4
107		max	0	1	.002	2	0	10	3.016e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.002	4	-1.097e-4	1	NC	1	NC	1
109		max	0	1	.001	2	0	10	3.016e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	0	4	-1.097e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	3.016e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-1.097e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	3.016e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-1.097e-4	1	NC	1	NC	1
115	M6	max	.006	2	.032	2	0	9	7.364e-4	4	NC	3	NC	1
116		min	-.011	3	-.031	3	-.009	5	-3.577e-7	9	1338.175	2	5375.729	3
117		max	.005	2	.03	2	0	9	7.592e-4	4	NC	3	NC	1
118		min	-.01	3	-.03	3	-.009	5	-7.808e-7	9	1434.927	2	5676.42	3
119		max	.005	2	.027	2	0	9	7.821e-4	4	NC	3	NC	1
120		min	-.009	3	-.028	3	-.009	5	-1.204e-6	9	1546.198	2	6038.687	3
121		max	.005	2	.025	2	0	9	8.049e-4	4	NC	3	NC	1
122		min	-.009	3	-.026	3	-.009	5	-1.627e-6	9	1674.902	2	6475.038	3
123		max	.005	2	.023	2	0	9	8.277e-4	4	NC	3	NC	1
124		min	-.008	3	-.025	3	-.009	5	-2.05e-6	9	1824.796	2	7002.224	3
125		max	.004	2	.021	2	0	9	8.505e-4	4	NC	3	NC	1
126		min	-.008	3	-.023	3	-.009	5	-2.473e-6	9	2000.798	2	7642.931	3
127		max	.004	2	.019	2	0	9	8.733e-4	4	NC	3	NC	1
128		min	-.007	3	-.021	3	-.008	5	-2.896e-6	9	2209.471	2	8428.37	3
129		max	.004	2	.017	2	0	9	8.962e-4	4	NC	3	NC	1
130		min	-.006	3	-.02	3	-.008	5	-3.32e-6	9	2459.763	2	9402.361	3
131		max	.003	2	.015	2	0	9	9.19e-4	4	NC	3	NC	1
132		min	-.006	3	-.018	3	-.008	5	-3.743e-6	9	2764.201	2	NC	1
133		max	.003	2	.014	2	0	9	9.418e-4	4	NC	3	NC	1
134		min	-.005	3	-.016	3	-.007	5	-4.166e-6	9	3140.867	2	NC	1
135		max	.003	2	.012	2	0	9	9.646e-4	4	NC	3	NC	1
136		min	-.005	3	-.014	3	-.007	5	-4.589e-6	9	3616.868	2	NC	1
137		max	.002	2	.01	2	0	9	9.874e-4	4	NC	3	NC	1
138		min	-.004	3	-.013	3	-.006	5	-5.012e-6	9	4234.767	2	NC	1
139		max	.002	2	.008	2	0	9	1.01e-3	4	NC	1	NC	1
140		min	-.004	3	-.011	3	-.005	5	-5.435e-6	9	5065.455	2	NC	1
141		max	.002	2	.007	2	0	9	1.033e-3	4	NC	1	NC	1
142		min	-.003	3	-.009	3	-.004	5	-5.858e-6	9	6236.452	2	NC	1
143		max	.001	2	.005	2	0	9	1.056e-3	4	NC	1	NC	1
144		min	-.002	3	-.007	3	-.004	5	-6.282e-6	9	8002.676	2	NC	1
145		max	0	2	.004	2	0	9	1.079e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.003	4	-6.705e-6	9	NC	1	NC	1
147		max	0	2	.003	2	0	9	1.102e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.002	4	-7.128e-6	9	NC	1	NC	1
149		max	0	2	.001	2	0	9	1.124e-3	4	NC	1	NC	1
150		min	0	3	-.002	3	0	4	-7.551e-6	9	NC	1	NC	1
151		max	0	1	0	1	0	1	1.147e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-7.974e-6	9	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	3.798e-6	9	NC	1	NC	1
154		min	0	1	0	1	0	1	-5.482e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.003	4	3.367e-6	9	NC	1	NC	1
156		min	0	2	-.002	3	0	9	-5.412e-4	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	2.935e-6	9	NC	1	NC	1
158			min	0	2	-.004	3	0	9	-5.343e-4	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.008	4	2.504e-6	9	NC	1	NC	1
160			min	-.001	2	-.006	3	0	9	-5.273e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.011	4	2.073e-6	9	NC	1	NC	1
162			min	-.002	2	-.008	3	0	9	-5.203e-4	4	9487.656	2	NC	1
163		6	max	.002	3	.006	2	.014	4	2.709e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	9	-5.134e-4	4	7584.262	2	NC	1
165		7	max	.002	3	.007	2	.016	4	5.569e-5	3	NC	1	NC	1
166			min	-.003	2	-.011	3	0	9	-5.064e-4	4	6281.136	2	NC	1
167		8	max	.003	3	.009	2	.019	4	8.428e-5	3	NC	1	NC	1
168			min	-.003	2	-.013	3	0	9	-4.995e-4	4	5325.304	2	NC	1
169		9	max	.003	3	.01	2	.021	4	1.129e-4	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	9	-4.925e-4	4	4590.675	2	NC	1
171		10	max	.003	3	.011	2	.023	4	1.415e-4	3	NC	3	NC	1
172			min	-.004	2	-.016	3	0	9	-4.856e-4	4	4007.275	2	NC	1
173		11	max	.004	3	.013	2	.026	4	1.701e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	-4.786e-4	4	3532.989	2	NC	1
175		12	max	.004	3	.015	2	.028	4	1.987e-4	3	NC	3	NC	1
176			min	-.005	2	-.019	3	0	9	-4.717e-4	4	3140.77	2	NC	1
177		13	max	.004	3	.016	2	.03	4	2.273e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	0	9	-4.647e-4	4	2812.296	2	NC	1
179		14	max	.005	3	.018	2	.032	4	2.559e-4	3	NC	3	NC	1
180			min	-.006	2	-.021	3	0	9	-4.577e-4	4	2534.592	2	NC	1
181		15	max	.005	3	.02	2	.033	4	2.845e-4	3	NC	3	NC	1
182			min	-.006	2	-.022	3	0	9	-4.508e-4	4	2298.125	2	NC	1
183		16	max	.006	3	.022	2	.035	4	3.131e-4	3	NC	3	NC	1
184			min	-.007	2	-.023	3	0	9	-4.438e-4	4	2095.683	2	NC	1
185		17	max	.006	3	.024	2	.037	4	3.417e-4	3	NC	3	NC	1
186			min	-.007	2	-.024	3	0	9	-4.369e-4	4	1921.675	2	NC	1
187		18	max	.006	3	.026	2	.038	4	3.703e-4	3	NC	3	NC	1
188			min	-.007	2	-.025	3	0	9	-4.299e-4	4	1771.69	2	NC	1
189		19	max	.007	3	.028	2	.04	4	3.989e-4	3	NC	3	NC	1
190			min	-.008	2	-.026	3	0	9	-4.23e-4	4	1642.201	2	NC	1
191	M8	1	max	.003	1	.037	2	0	9	2.893e-3	4	NC	1	NC	1
192			min	0	15	-.031	3	-.042	4	-2.782e-4	3	NC	1	464.924	4
193		2	max	.003	1	.035	2	0	9	2.893e-3	4	NC	1	NC	1
194			min	0	15	-.029	3	-.038	4	-2.782e-4	3	NC	1	506.742	4
195		3	max	.002	1	.033	2	0	9	2.893e-3	4	NC	1	NC	1
196			min	0	15	-.028	3	-.035	4	-2.782e-4	3	NC	1	556.504	4
197		4	max	.002	1	.031	2	0	9	2.893e-3	4	NC	1	NC	1
198			min	0	15	-.026	3	-.031	4	-2.782e-4	3	NC	1	616.301	4
199		5	max	.002	1	.029	2	0	9	2.893e-3	4	NC	1	NC	1
200			min	0	15	-.024	3	-.028	4	-2.782e-4	3	NC	1	688.982	4
201		6	max	.002	1	.027	2	0	9	2.893e-3	4	NC	1	NC	1
202			min	0	15	-.022	3	-.025	4	-2.782e-4	3	NC	1	778.509	4
203		7	max	.002	1	.024	2	0	9	2.893e-3	4	NC	1	NC	1
204			min	0	15	-.021	3	-.022	4	-2.782e-4	3	NC	1	890.524	4
205		8	max	.002	1	.022	2	0	9	2.893e-3	4	NC	1	NC	1
206			min	0	15	-.019	3	-.019	4	-2.782e-4	3	NC	1	1033.289	4
207		9	max	.002	1	.02	2	0	9	2.893e-3	4	NC	1	NC	1
208			min	0	15	-.017	3	-.016	4	-2.782e-4	3	NC	1	1219.297	4
209		10	max	.001	1	.018	2	0	9	2.893e-3	4	NC	1	NC	1
210			min	0	15	-.016	3	-.013	4	-2.782e-4	3	NC	1	1468.185	4
211		11	max	.001	1	.016	2	0	9	2.893e-3	4	NC	1	NC	1
212			min	0	15	-.014	3	-.011	4	-2.782e-4	3	NC	1	1812.294	4
213		12	max	.001	1	.014	2	0	9	2.893e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214		min	0	15	-.012	3	-.008	4	-2.782e-4	3	NC	1	2308.031	4
215		max	0	1	.012	2	0	9	2.893e-3	4	NC	1	NC	1
216		min	0	15	-.01	3	-.006	4	-2.782e-4	3	NC	1	3061.188	4
217		max	0	1	.01	2	0	9	2.893e-3	4	NC	1	NC	1
218		min	0	15	-.009	3	-.005	4	-2.782e-4	3	NC	1	4289.967	4
219		max	0	1	.008	2	0	9	2.893e-3	4	NC	1	NC	1
220		min	0	15	-.007	3	-.003	4	-2.782e-4	3	NC	1	6506.949	4
221		max	0	1	.006	2	0	9	2.893e-3	4	NC	1	NC	1
222		min	0	15	-.005	3	-.002	4	-2.782e-4	3	NC	1	NC	1
223		max	0	1	.004	2	0	9	2.893e-3	4	NC	1	NC	1
224		min	0	15	-.003	3	0	4	-2.782e-4	3	NC	1	NC	1
225		max	0	1	.002	2	0	9	2.893e-3	4	NC	1	NC	1
226		min	0	15	-.002	3	0	4	-2.782e-4	3	NC	1	NC	1
227		max	0	1	0	1	0	1	2.893e-3	4	NC	1	NC	1
228		min	0	1	0	1	0	1	-2.782e-4	3	NC	1	NC	1
229	M10	max	.002	2	.011	2	0	10	1.598e-4	1	NC	3	NC	1
230		min	-.003	3	-.011	3	-.005	4	-6.495e-4	3	3914.594	2	NC	1
231		max	.002	2	.01	2	0	10	1.522e-4	1	NC	3	NC	1
232		min	-.002	3	-.011	3	-.006	4	-6.258e-4	3	4281.136	2	NC	1
233		max	.002	2	.009	2	0	10	1.902e-4	4	NC	3	NC	1
234		min	-.002	3	-.01	3	-.006	4	-6.021e-4	3	4718.666	2	NC	1
235		max	.002	2	.008	2	0	10	2.381e-4	4	NC	1	NC	1
236		min	-.002	3	-.01	3	-.006	4	-5.784e-4	3	5244.518	2	NC	1
237		max	.002	2	.007	2	0	10	2.859e-4	4	NC	1	NC	1
238		min	-.002	3	-.009	3	-.006	4	-5.547e-4	3	5881.853	2	NC	1
239		max	.001	2	.006	2	0	3	3.338e-4	4	NC	1	NC	1
240		min	-.002	3	-.009	3	-.006	4	-5.31e-4	3	6662.112	2	NC	1
241		max	.001	2	.006	2	0	3	3.816e-4	4	NC	1	NC	1
242		min	-.002	3	-.008	3	-.006	4	-5.073e-4	3	7628.785	2	NC	1
243		max	.001	2	.005	2	0	3	4.295e-4	4	NC	1	NC	1
244		min	-.002	3	-.008	3	-.006	4	-4.836e-4	3	8843.358	2	NC	1
245		max	.001	2	.004	2	0	3	4.773e-4	4	NC	1	NC	1
246		min	-.001	3	-.007	3	-.006	4	-4.599e-4	3	NC	1	NC	1
247		max	0	2	.003	2	0	3	5.252e-4	4	NC	1	NC	1
248		min	-.001	3	-.007	3	-.005	4	-4.362e-4	3	NC	1	NC	1
249		max	0	2	.003	2	0	3	5.73e-4	4	NC	1	NC	1
250		min	-.001	3	-.006	3	-.005	4	-4.125e-4	3	NC	1	NC	1
251		max	0	2	.002	2	0	3	6.209e-4	4	NC	1	NC	1
252		min	0	3	-.005	3	-.005	4	-3.888e-4	3	NC	1	NC	1
253		max	0	2	.002	2	0	3	6.687e-4	4	NC	1	NC	1
254		min	0	3	-.005	3	-.004	4	-3.651e-4	3	NC	1	NC	1
255		max	0	2	.001	2	0	3	7.165e-4	4	NC	1	NC	1
256		min	0	3	-.004	3	-.004	4	-3.414e-4	3	NC	1	NC	1
257		max	0	2	0	2	0	3	7.644e-4	4	NC	1	NC	1
258		min	0	3	-.003	3	-.003	4	-3.177e-4	3	NC	1	NC	1
259		max	0	2	0	2	0	3	8.122e-4	4	NC	1	NC	1
260		min	0	3	-.002	3	-.002	4	-2.94e-4	3	NC	1	NC	1
261		max	0	2	0	2	0	3	8.601e-4	4	NC	1	NC	1
262		min	0	3	-.002	3	-.001	4	-2.703e-4	3	NC	1	NC	1
263		max	0	2	0	2	0	3	9.079e-4	4	NC	1	NC	1
264		min	0	3	0	3	0	4	-2.466e-4	3	NC	1	NC	1
265		max	0	1	0	1	0	1	9.558e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	-2.229e-4	3	NC	1	NC	1
267	M11	max	0	1	0	1	0	1	1.068e-4	3	NC	1	NC	1
268		min	0	1	0	1	0	1	-4.57e-4	4	NC	1	NC	1
269		max	0	3	0	2	.002	4	7.996e-5	3	NC	1	NC	1
270		min	0	2	0	3	0	3	-4.857e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.005	4	5.315e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-5.144e-4	4	NC	1	NC	1
273	4	max	0	3	0	2	.007	4	2.635e-5	3	NC	1	NC	1
274		min	0	2	-.003	3	-.001	3	-5.431e-4	4	NC	1	NC	1
275	5	max	0	3	0	2	.009	4	5.737e-6	10	NC	1	NC	1
276		min	0	2	-.004	3	-.002	3	-5.718e-4	4	NC	1	NC	1
277	6	max	0	3	0	2	.012	4	6.611e-6	10	NC	1	NC	1
278		min	0	2	-.005	3	-.002	3	-6.005e-4	4	NC	1	NC	1
279	7	max	0	3	0	2	.014	4	7.485e-6	10	NC	1	NC	1
280		min	0	2	-.005	3	-.002	3	-6.292e-4	4	NC	1	NC	1
281	8	max	0	3	.001	2	.016	5	8.36e-6	10	NC	1	NC	1
282		min	-.001	2	-.006	3	-.003	3	-6.58e-4	4	NC	1	NC	1
283	9	max	.001	3	.001	2	.018	5	9.234e-6	10	NC	1	NC	1
284		min	-.001	2	-.007	3	-.003	3	-6.867e-4	4	NC	1	NC	1
285	10	max	.001	3	.002	2	.02	5	1.011e-5	10	NC	1	NC	1
286		min	-.001	2	-.007	3	-.003	3	-7.154e-4	4	NC	1	NC	1
287	11	max	.001	3	.002	2	.022	5	1.098e-5	10	NC	1	NC	1
288		min	-.002	2	-.008	3	-.003	3	-7.441e-4	4	NC	1	NC	1
289	12	max	.002	3	.003	2	.024	5	1.186e-5	10	NC	1	NC	1
290		min	-.002	2	-.008	3	-.003	3	-7.728e-4	4	NC	1	NC	1
291	13	max	.002	3	.004	2	.026	5	1.273e-5	10	NC	1	NC	1
292		min	-.002	2	-.008	3	-.003	3	-8.015e-4	4	NC	1	NC	1
293	14	max	.002	3	.005	2	.027	5	1.361e-5	10	NC	1	NC	1
294		min	-.002	2	-.009	3	-.003	3	-8.302e-4	4	NC	1	NC	1
295	15	max	.002	3	.005	2	.029	5	1.448e-5	10	NC	1	NC	1
296		min	-.002	2	-.009	3	-.003	3	-8.59e-4	4	8508.311	2	NC	1
297	16	max	.002	3	.006	2	.031	5	1.535e-5	10	NC	1	NC	1
298		min	-.002	2	-.009	3	-.003	3	-8.877e-4	4	7226.41	2	NC	1
299	17	max	.002	3	.007	2	.032	5	1.623e-5	10	NC	1	NC	1
300		min	-.002	2	-.009	3	-.003	3	-9.164e-4	4	6231.445	2	NC	1
301	18	max	.002	3	.008	2	.034	5	1.71e-5	10	NC	1	NC	1
302		min	-.003	2	-.009	3	-.003	3	-9.451e-4	4	5450.63	2	NC	1
303	19	max	.003	3	.01	2	.035	5	1.798e-5	10	NC	3	NC	1
304		min	-.003	2	-.009	3	-.002	3	-9.738e-4	4	4832.552	2	NC	1
305	M12	1	max	0	.012	2	.001	3	3.404e-3	4	NC	1	NC	1
306		min	0	5	-.011	3	-.038	5	-2.275e-5	10	NC	1	502.481	5
307	2	max	0	1	.012	2	.001	3	3.404e-3	4	NC	1	NC	1
308		min	0	5	-.011	3	-.035	5	-2.275e-5	10	NC	1	547.661	5
309	3	max	0	1	.011	2	.001	3	3.404e-3	4	NC	1	NC	1
310		min	0	5	-.01	3	-.032	5	-2.275e-5	10	NC	1	601.422	5
311	4	max	0	1	.01	2	.001	3	3.404e-3	4	NC	1	NC	1
312		min	0	5	-.009	3	-.029	5	-2.275e-5	10	NC	1	666.021	5
313	5	max	0	1	.01	2	0	3	3.404e-3	4	NC	1	NC	1
314		min	0	5	-.009	3	-.026	5	-2.275e-5	10	NC	1	744.538	5
315	6	max	0	1	.009	2	0	3	3.404e-3	4	NC	1	NC	1
316		min	0	5	-.008	3	-.023	5	-2.275e-5	10	NC	1	841.251	5
317	7	max	0	1	.008	2	0	3	3.404e-3	4	NC	1	NC	1
318		min	0	5	-.008	3	-.02	5	-2.275e-5	10	NC	1	962.253	5
319	8	max	0	1	.008	2	0	3	3.404e-3	4	NC	1	NC	1
320		min	0	5	-.007	3	-.017	5	-2.275e-5	10	NC	1	1116.468	5
321	9	max	0	1	.007	2	0	3	3.404e-3	4	NC	1	NC	1
322		min	0	5	-.006	3	-.015	5	-2.275e-5	10	NC	1	1317.389	5
323	10	max	0	1	.006	2	0	3	3.404e-3	4	NC	1	NC	1
324		min	0	5	-.006	3	-.012	5	-2.275e-5	10	NC	1	1586.226	5
325	11	max	0	1	.006	2	0	3	3.404e-3	4	NC	1	NC	1
326		min	0	5	-.005	3	-.01	5	-2.275e-5	10	NC	1	1957.905	5
327	12	max	0	1	.005	2	0	3	3.404e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	5	-.004	3	-.008	5	-2.275e-5	10	NC	1	2493.345	5
329		max	0	1	.004	2	0	3	3.404e-3	4	NC	1	NC	1
330		min	0	5	-.004	3	-.006	5	-2.275e-5	10	NC	1	3306.802	5
331		max	0	1	.003	2	0	3	3.404e-3	4	NC	1	NC	1
332		min	0	5	-.003	3	-.004	5	-2.275e-5	10	NC	1	4633.925	5
333		max	0	1	.003	2	0	3	3.404e-3	4	NC	1	NC	1
334		min	0	5	-.003	3	-.003	5	-2.275e-5	10	NC	1	7028.274	5
335		max	0	1	.002	2	0	3	3.404e-3	4	NC	1	NC	1
336		min	0	5	-.002	3	-.002	5	-2.275e-5	10	NC	1	NC	1
337		max	0	1	.001	2	0	3	3.404e-3	4	NC	1	NC	1
338		min	0	5	-.001	3	0	5	-2.275e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	3	3.404e-3	4	NC	1	NC	1
340		min	0	5	0	3	0	5	-2.275e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.404e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	-2.275e-5	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.006	5	4.364e-3	2	NC	1	NC	1
344		min	-.01	2	-.022	2	0	9	-6.513e-3	3	NC	1	NC	1
345		max	.01	3	.016	3	.008	5	2.166e-3	2	NC	4	NC	1
346		min	-.01	2	-.013	2	0	9	-3.205e-3	3	5474.269	2	NC	1
347		max	.01	3	.007	3	.009	5	2.549e-4	5	NC	4	NC	1
348		min	-.01	2	-.005	2	-.001	9	-7.465e-5	9	2805.878	3	NC	1
349		max	.01	3	.002	2	.011	5	2.566e-4	5	NC	4	NC	1
350		min	-.01	2	-.002	3	-.002	9	-6.366e-5	9	1876.675	3	9779.241	5
351		max	.01	3	.009	2	.013	5	2.582e-4	5	NC	4	NC	1
352		min	-.01	2	-.009	3	-.002	9	-5.267e-5	9	1468.498	3	6896.256	5
353		max	.01	3	.014	2	.015	5	2.599e-4	5	NC	4	NC	1
354		min	-.01	2	-.014	3	-.002	9	-4.168e-5	9	1248.419	3	5238.292	5
355		max	.01	3	.018	2	.017	5	2.615e-4	5	NC	4	NC	1
356		min	-.01	2	-.018	3	-.001	9	-3.069e-5	9	1119.368	3	4179.209	5
357		max	.01	3	.022	2	.02	5	2.632e-4	5	NC	4	NC	1
358		min	-.01	2	-.022	3	-.001	9	-1.97e-5	9	1043.462	3	3454.266	5
359		max	.01	3	.024	2	.022	5	2.649e-4	5	NC	4	NC	1
360		min	-.01	2	-.023	3	0	9	-8.705e-6	9	1003.695	3	2933.164	5
361		max	.009	3	.025	2	.025	4	2.696e-4	4	NC	4	NC	1
362		min	-.01	2	-.024	3	0	9	-1.72e-6	10	992.434	3	2532.083	4
363		max	.009	3	.025	2	.027	4	2.751e-4	4	NC	4	NC	1
364		min	-.01	2	-.023	3	0	10	-4.331e-6	10	1007.432	3	2222.048	4
365		max	.009	3	.023	2	.03	4	2.807e-4	4	NC	4	NC	1
366		min	-.01	2	-.021	3	0	10	-6.942e-6	10	1050.771	3	1982.65	4
367		max	.009	3	.02	2	.032	4	2.862e-4	4	NC	4	NC	1
368		min	-.01	2	-.018	3	0	10	-9.553e-6	10	1129.72	3	1794.733	4
369		max	.009	3	.015	2	.035	4	2.918e-4	4	NC	4	NC	1
370		min	-.01	2	-.014	3	0	10	-1.216e-5	10	1260.267	3	1645.502	4
371		max	.009	3	.009	2	.037	4	2.973e-4	4	NC	4	NC	1
372		min	-.01	2	-.008	3	0	10	-1.478e-5	10	1477.121	3	1526.164	4
373		max	.009	3	.002	2	.039	4	4.541e-4	4	NC	4	NC	1
374		min	-.01	2	-.002	3	0	10	-1.668e-5	10	1865.611	3	1430.534	4
375		max	.009	3	.006	3	.041	4	4.208e-3	4	NC	4	NC	1
376		min	-.01	2	-.008	2	0	10	-5.336e-6	9	2707.012	3	1354.315	4
377		max	.009	3	.015	3	.042	4	3.233e-3	2	NC	1	NC	1
378		min	-.01	2	-.018	2	0	10	-1.878e-3	3	5307.93	3	1294.068	4
379		max	.009	3	.023	3	.044	4	6.526e-3	2	NC	1	NC	1
380		min	-.01	2	-.03	2	0	9	-3.899e-3	3	5693.919	2	1248.959	4
381	M5	max	.027	3	.079	3	.006	5	2.525e-5	4	NC	1	NC	1
382		min	-.029	2	-.064	2	0	9	8.013e-8	11	4054.867	3	NC	1
383		max	.027	3	.048	3	.007	5	1.739e-4	3	NC	4	NC	1
384		min	-.029	2	-.039	2	0	9	-4.981e-6	9	1833.931	2	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.027	3	.019	3	.009	5	3.263e-4	3	NC	4	NC	1
386		min	-.029	2	-.014	2	0	9	-9.963e-6	9	939.979	2	NC	1
387	4	max	.027	3	.007	2	.011	5	3.128e-4	3	NC	5	NC	1
388		min	-.029	2	-.006	3	0	9	-9.45e-6	9	641.724	3	9693.742	3
389	5	max	.027	3	.026	2	.013	5	2.993e-4	3	NC	5	NC	1
390		min	-.029	2	-.026	3	0	9	-8.938e-6	9	501.555	3	8120.187	3
391	6	max	.027	3	.042	2	.016	5	2.858e-4	3	NC	5	NC	1
392		min	-.029	2	-.043	3	0	9	-8.425e-6	9	426.507	3	7345.555	3
393	7	max	.026	3	.055	2	.018	5	2.723e-4	3	NC	5	NC	1
394		min	-.029	2	-.055	3	0	9	-7.913e-6	9	382.784	3	6999.833	3
395	8	max	.026	3	.065	2	.021	4	2.701e-4	5	NC	5	NC	1
396		min	-.029	2	-.064	3	0	9	-7.401e-6	9	357.297	3	6941.948	3
397	9	max	.026	3	.072	2	.023	4	2.79e-4	5	NC	5	NC	1
398		min	-.029	2	-.069	3	0	9	-6.888e-6	9	344.205	3	7120.433	3
399	10	max	.026	3	.075	2	.026	4	2.881e-4	4	NC	5	NC	1
400		min	-.029	2	-.07	3	0	9	-6.376e-6	9	337.914	2	7532.48	3
401	11	max	.026	3	.074	2	.029	4	2.973e-4	4	NC	5	NC	1
402		min	-.029	2	-.068	3	0	9	-5.863e-6	9	340.273	2	8214.582	3
403	12	max	.026	3	.069	2	.031	4	3.065e-4	4	NC	5	NC	1
404		min	-.029	2	-.062	3	0	9	-5.351e-6	9	352.944	2	9250.66	3
405	13	max	.026	3	.06	2	.034	4	3.156e-4	4	NC	5	NC	1
406		min	-.029	2	-.053	3	0	9	-4.838e-6	9	378.973	2	NC	1
407	14	max	.026	3	.046	2	.036	4	3.248e-4	4	NC	5	NC	1
408		min	-.028	2	-.04	3	0	9	-4.326e-6	9	425.361	2	NC	1
409	15	max	.026	3	.028	2	.038	4	3.34e-4	4	NC	5	NC	1
410		min	-.028	2	-.024	3	0	9	-3.813e-6	9	508.675	2	NC	1
411	16	max	.026	3	.005	2	.04	4	4.923e-4	4	NC	5	NC	1
412		min	-.028	2	-.005	3	0	9	-3.784e-6	9	646.925	3	NC	1
413	17	max	.026	3	.017	3	.042	4	4.199e-3	4	NC	4	NC	1
414		min	-.029	2	-.023	2	0	9	-1.524e-5	9	938.331	3	NC	1
415	18	max	.026	3	.042	3	.043	4	2.157e-3	4	NC	4	NC	1
416		min	-.028	2	-.055	2	0	9	-7.856e-6	9	1839.987	3	NC	1
417	19	max	.026	3	.067	3	.044	4	7.264e-6	5	NC	3	NC	1
418		min	-.028	2	-.089	2	0	9	-3.281e-6	3	1895.181	2	NC	1
419	M9	1	max	.01	.025	3	.006	5	6.55e-3	3	NC	1	NC	1
420		min	-.01	2	-.022	2	0	9	-4.364e-3	2	NC	1	NC	1
421	2	max	.01	3	.015	3	.005	4	3.203e-3	3	NC	4	NC	1
422		min	-.01	2	-.013	2	0	10	-2.165e-3	2	5474.764	2	NC	1
423	3	max	.01	3	.005	3	.006	4	7.45e-5	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	10	-8.197e-5	3	2575.369	3	NC	1
425	4	max	.01	3	.002	2	.006	4	6.218e-5	1	NC	4	NC	1
426		min	-.01	2	-.003	3	0	3	-8.455e-5	3	1777.198	3	NC	1
427	5	max	.01	3	.009	2	.007	4	4.986e-5	1	NC	4	NC	1
428		min	-.01	2	-.01	3	-.002	3	-8.713e-5	3	1410.914	3	8084.42	3
429	6	max	.01	3	.014	2	.008	4	3.753e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.003	3	-8.971e-5	3	1209.474	3	7014.685	3
431	7	max	.01	3	.018	2	.01	4	2.521e-5	1	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-9.23e-5	3	1090.298	3	6394.24	3
433	8	max	.01	3	.022	2	.012	4	1.289e-5	1	NC	4	NC	1
434		min	-.01	2	-.022	3	-.005	3	-9.488e-5	3	1020.185	3	6046.415	3
435	9	max	.01	3	.024	2	.015	4	2.096e-5	5	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-9.746e-5	3	984.014	3	5160.849	4
437	10	max	.01	3	.025	2	.017	4	3.284e-5	5	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-1.e-4	3	975.021	3	4009.533	4
439	11	max	.01	3	.025	2	.02	5	4.472e-5	5	NC	4	NC	1
440		min	-.01	2	-.024	3	-.005	3	-1.026e-4	3	991.387	3	3237.825	4
441	12	max	.01	3	.023	2	.023	5	5.661e-5	5	NC	4	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.01	2	-.022	3	-.005	3	-1.052e-4	3	1035.4	3	2680.496	5
443		13	max	.009	3	.02	2	.026	5	6.849e-5	5	NC	4	NC	1
444			min	-.01	2	-.018	3	-.005	3	-1.078e-4	3	1114.387	3	2277.685	5
445		14	max	.009	3	.015	2	.029	5	8.037e-5	5	NC	4	NC	1
446			min	-.01	2	-.014	3	-.004	3	-1.104e-4	3	1244.254	3	1978.63	5
447		15	max	.009	3	.009	2	.033	5	9.225e-5	5	NC	4	NC	1
448			min	-.01	2	-.008	3	-.003	3	-1.13e-4	3	1459.404	3	1751.213	5
449		16	max	.009	3	.002	2	.036	5	2.65e-4	5	NC	4	NC	1
450			min	-.01	2	-.002	3	-.002	3	-1.046e-4	3	1844.301	3	1575.161	5
451		17	max	.009	3	.006	3	.038	5	4.276e-3	4	NC	4	NC	1
452			min	-.01	2	-.008	2	-.001	3	-2.514e-5	9	2677.139	3	1437.081	5
453		18	max	.009	3	.015	3	.041	5	2.135e-3	5	NC	1	NC	1
454			min	-.01	2	-.018	2	0	9	-3.233e-3	2	5250.51	3	1326.715	4
455		19	max	.009	3	.024	3	.044	4	3.893e-3	3	NC	1	NC	1
456			min	-.01	2	-.03	2	0	9	-6.527e-3	2	5707.781	2	1233.706	4
457	M13	1	max	0	9	.025	3	.01	3	3.92e-3	3	NC	1	NC	1
458			min	-.006	5	-.022	2	-.01	2	-3.253e-3	2	NC	1	NC	1
459		2	max	0	9	.053	3	.008	3	4.793e-3	3	NC	4	NC	1
460			min	-.006	5	-.04	2	-.01	2	-3.971e-3	2	2837.834	3	NC	1
461		3	max	0	9	.077	3	.008	3	5.666e-3	3	NC	4	NC	1
462			min	-.006	5	-.056	2	-.011	2	-4.688e-3	2	1523.768	3	NC	1
463		4	max	0	9	.094	3	.01	3	6.538e-3	3	NC	4	NC	1
464			min	-.006	5	-.068	2	-.012	2	-5.405e-3	2	1138.747	3	NC	1
465		5	max	0	9	.104	3	.012	3	7.411e-3	3	NC	4	NC	1
466			min	-.006	5	-.075	2	-.015	2	-6.122e-3	2	999.09	3	NC	1
467		6	max	0	9	.106	3	.015	3	8.284e-3	3	NC	4	NC	1
468			min	-.006	5	-.078	2	-.018	2	-6.84e-3	2	976.805	3	9653.977	2
469		7	max	0	9	.101	3	.018	3	9.157e-3	3	NC	4	NC	1
470			min	-.006	5	-.076	2	-.021	2	-7.557e-3	2	1039.238	3	6731.445	2
471		8	max	0	9	.092	3	.021	3	1.003e-2	3	NC	4	NC	1
472			min	-.006	5	-.072	2	-.025	2	-8.274e-3	2	1178.764	3	5190.966	2
473		9	max	0	9	.083	3	.024	3	1.09e-2	3	NC	4	NC	4
474			min	-.006	5	-.067	2	-.028	2	-8.991e-3	2	1369.195	3	4398.705	2
475		10	max	0	9	.079	3	.027	3	1.178e-2	3	NC	4	NC	4
476			min	-.006	5	-.064	2	-.029	2	-9.709e-3	2	1486.023	3	4135.351	2
477		11	max	0	9	.083	3	.028	3	1.091e-2	3	NC	4	NC	4
478			min	-.006	5	-.067	2	-.028	2	-8.992e-3	2	1369.193	3	4176.838	3
479		12	max	0	9	.092	3	.029	3	1.004e-2	3	NC	4	NC	1
480			min	-.006	5	-.072	2	-.025	2	-8.274e-3	2	1178.762	3	4159.886	3
481		13	max	0	9	.101	3	.027	3	9.169e-3	3	NC	4	NC	1
482			min	-.006	5	-.076	2	-.021	2	-7.557e-3	2	1039.236	3	4447.65	3
483		14	max	0	9	.106	3	.025	3	8.301e-3	3	NC	4	NC	1
484			min	-.006	5	-.078	2	-.018	2	-6.84e-3	2	976.804	3	5098.27	3
485		15	max	0	9	.105	3	.022	3	7.432e-3	3	NC	4	NC	1
486			min	-.006	5	-.075	2	-.015	2	-6.123e-3	2	999.089	3	6326.295	3
487		16	max	0	9	.095	3	.019	3	6.564e-3	3	NC	4	NC	1
488			min	-.006	5	-.068	2	-.012	2	-5.405e-3	2	1138.746	3	8715.343	3
489		17	max	0	9	.078	3	.015	3	5.695e-3	3	NC	4	NC	1
490			min	-.006	5	-.056	2	-.011	2	-4.688e-3	2	1523.766	3	NC	1
491		18	max	0	9	.054	3	.012	3	4.827e-3	3	NC	4	NC	1
492			min	-.006	5	-.04	2	-.01	2	-3.971e-3	2	2837.831	3	NC	1
493		19	max	0	9	.027	3	.01	3	3.958e-3	3	NC	1	NC	1
494			min	-.006	5	-.022	2	-.01	2	-3.254e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.024	3	.009	3	4.315e-3	2	NC	1	NC	1
496			min	-.044	4	-.03	2	-.01	2	-3.4e-3	3	NC	1	NC	1
497		2	max	0	9	.04	3	.012	3	5.268e-3	2	NC	4	NC	1
498			min	-.044	4	-.057	2	-.01	2	-4.1e-3	3	2835.512	2	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556			min	-.004	4	-.007	3	-.037	3	-3.641e-3	2	7519.447	1	995.755	3
557		13	max	.002	3	.006	5	.027	2	4.462e-3	3	NC	2	6130.428	9
558			min	-.005	4	-.007	3	-.036	3	-3.969e-3	2	8143.167	1	986.138	3
559		14	max	.002	3	.006	5	.024	2	4.8e-3	3	NC	2	6366.538	9
560			min	-.005	4	-.006	3	-.032	3	-4.297e-3	2	9182.439	1	1017.108	3
561		15	max	.002	3	.006	5	.018	2	5.137e-3	3	NC	1	8274.271	15
562			min	-.005	4	-.006	3	-.026	3	-4.625e-3	2	NC	1	1104.503	3
563		16	max	.002	3	.006	5	.011	1	5.475e-3	3	NC	1	NC	13
564			min	-.006	4	-.005	3	-.017	3	-4.952e-3	2	NC	1	1291.291	3
565		17	max	.002	3	.006	2	.006	4	5.813e-3	3	NC	1	NC	4
566			min	-.006	4	-.004	3	-.004	3	-5.28e-3	2	8926.787	2	1712.231	3
567		18	max	.002	3	.008	2	.013	3	6.15e-3	3	NC	1	NC	4
568			min	-.006	4	-.003	3	-.013	2	-5.608e-3	2	6840.493	2	3048.992	3
569		19	max	.002	3	.01	2	.034	3	6.488e-3	3	NC	1	NC	1
570			min	-.007	4	-.002	3	-.03	2	-5.936e-3	2	5518.026	2	NC	1
571	M16A	1	max	.002	2	.004	2	.01	3	1.827e-3	3	NC	1	NC	1
572			min	-.003	4	-.004	4	-.01	2	-1.997e-3	2	NC	1	NC	1
573		2	max	.001	2	.002	2	.003	3	1.765e-3	3	NC	1	NC	1
574			min	-.002	4	-.007	4	-.005	2	-1.905e-3	2	NC	1	8374.276	3
575		3	max	.001	2	0	2	.002	1	1.702e-3	3	NC	1	NC	4
576			min	-.002	4	-.01	4	-.006	5	-1.813e-3	2	9778.475	4	4746.157	3
577		4	max	.001	2	-.001	2	.004	1	1.64e-3	3	NC	1	NC	9
578			min	-.002	4	-.012	4	-.01	5	-1.721e-3	2	6708.605	4	3616.758	3
579		5	max	.001	2	-.002	2	.006	1	1.577e-3	3	NC	1	NC	9
580			min	-.002	4	-.014	4	-.015	5	-1.629e-3	2	5234.793	4	3130.525	3
581		6	max	.001	2	-.004	10	.007	1	1.515e-3	3	NC	3	NC	9
582			min	-.002	4	-.016	4	-.019	5	-1.537e-3	2	4405.631	4	2922.518	3
583		7	max	.001	2	-.004	10	.008	1	1.453e-3	3	NC	3	NC	9
584			min	-.002	4	-.018	4	-.023	5	-1.445e-3	2	3907	4	2693.567	5
585		8	max	0	2	-.005	10	.008	1	1.39e-3	3	NC	3	NC	9
586			min	-.002	4	-.019	4	-.026	5	-1.353e-3	2	3607.746	4	2319.952	5
587		9	max	0	2	-.005	12	.007	1	1.328e-3	3	NC	12	NC	9
588			min	-.001	4	-.019	4	-.029	5	-1.261e-3	2	3446.67	4	2112.313	5
589		10	max	0	2	-.005	12	.007	1	1.265e-3	3	NC	12	NC	9
590			min	-.001	4	-.019	4	-.03	5	-1.169e-3	2	3395.714	4	2018.762	5
591		11	max	0	2	-.005	12	.006	1	1.203e-3	3	NC	12	NC	9
592			min	-.001	4	-.019	4	-.03	5	-1.077e-3	2	3446.67	4	2018.954	5
593		12	max	0	2	-.005	12	.005	1	1.14e-3	3	NC	3	NC	9
594			min	0	4	-.018	4	-.028	5	-9.851e-4	2	3607.746	4	2113.342	5
595		13	max	0	2	-.004	12	.004	1	1.078e-3	3	NC	3	NC	1
596			min	0	4	-.016	4	-.026	5	-8.932e-4	2	3907	4	2323.629	5
597		14	max	0	2	-.004	12	.002	1	1.016e-3	3	NC	3	NC	1
598			min	0	4	-.014	4	-.022	5	-8.012e-4	2	4405.631	4	2705.081	5
599		15	max	0	2	-.003	12	.001	9	9.532e-4	3	NC	1	NC	1
600			min	0	4	-.012	4	-.018	5	-7.092e-4	2	5234.793	4	3384.963	5
601		16	max	0	2	-.002	12	0	9	8.908e-4	3	NC	1	NC	1
602			min	0	4	-.009	4	-.013	5	-6.172e-4	2	6708.605	4	4686.619	5
603		17	max	0	2	-.002	12	0	9	8.505e-4	4	NC	1	NC	1
604			min	0	4	-.006	4	-.008	5	-5.252e-4	2	9778.475	4	7648.253	5
605		18	max	0	2	0	12	0	3	9.103e-4	4	NC	1	NC	1
606			min	0	4	-.003	4	-.003	5	-4.332e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	9.7e-4	4	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.413e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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