

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II
Peak Velocity Pressure, $q_z$ =	26.53 psf	Including the gust factor, $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	1.67	$C_s$ = 0.8	
$S_1$ =	1.00	$\rho$ = 1.3	
$S_{D1}$ =	1.00	$\Omega$ = 1.25	
$T_a$ =	0.04	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

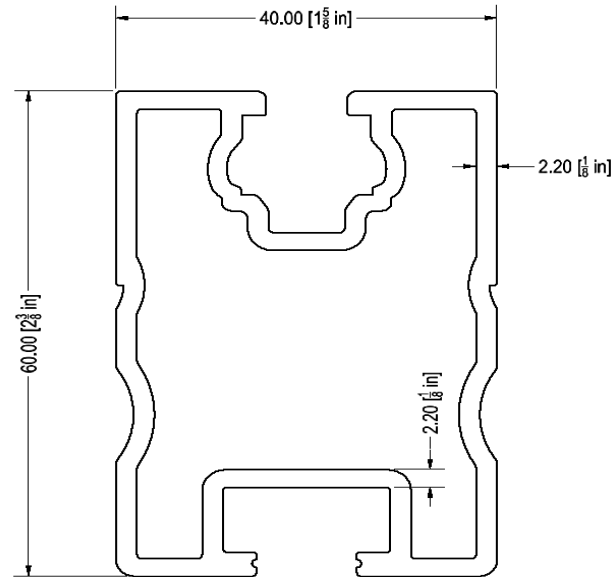
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

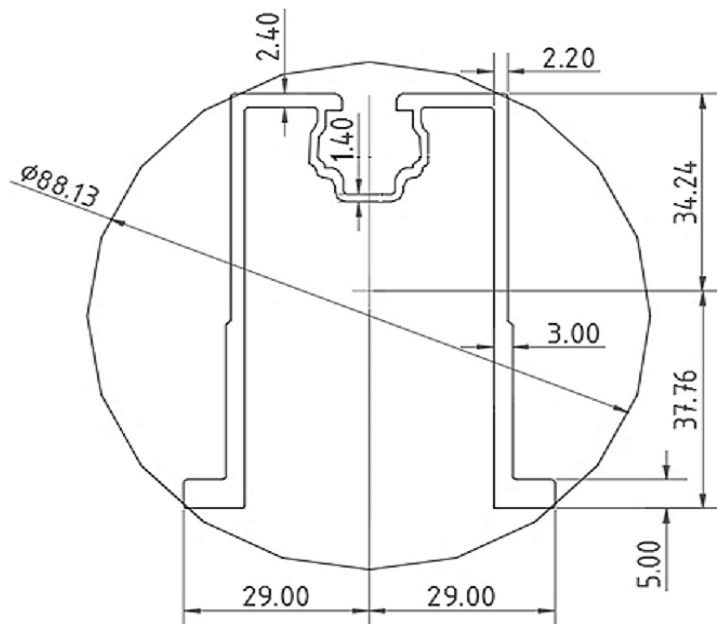
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42 in
$\Phi F_{ty}$ STRONG-AXIS =	29.99 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	-0.349 k-ft
$M_z$ =	-0.016 k-ft
$M_{y \text{ allowable}}$ =	1.276 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>29%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.76 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	-0.469 k-ft
$M_z$ =	-0.022 k-ft
$P_n$ =	0.156 k
$M_{y \text{ allowable}}$ =	1.460 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>38%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.023 k-ft
$P_n$ =	0.129 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>6%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

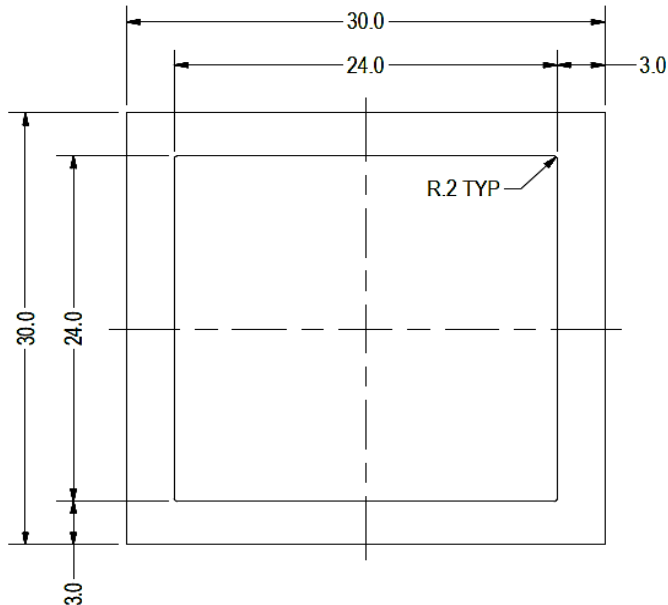
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.452 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>12%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

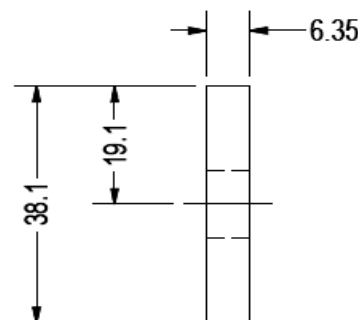
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.639 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<b>11%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.159 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>6%</b>



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

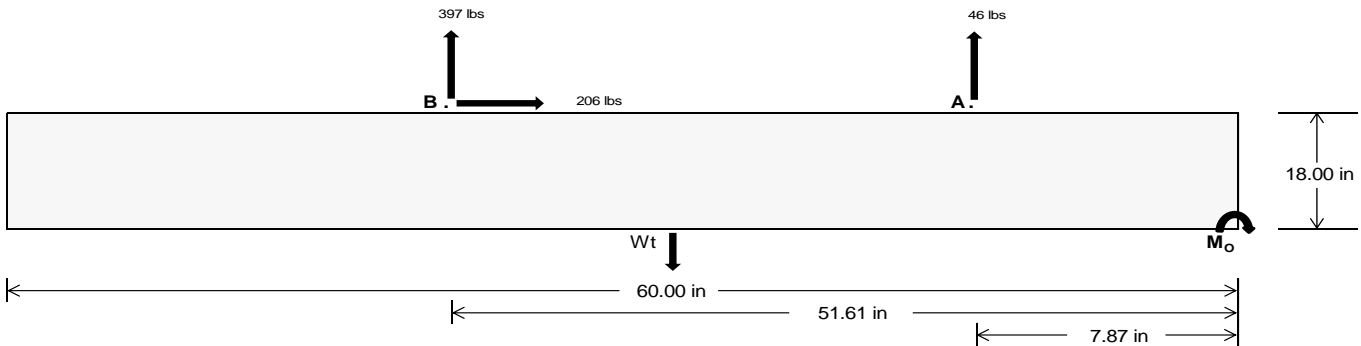
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>192.42</b>	<b>1652.39</b>	k
Compressive Load =	<b>1036.77</b>	<b>1059.76</b>	k
Lateral Load =	<b>18.85</b>	<b>856.39</b>	k
Moment (Weak Axis) =	<b>0.03</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 24546.4$  in-lbs  
Resisting Force Required = 818.21 lbs  
S.F. = 1.67  
Weight Required = 1363.69 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 205.85 lbs  
Friction = 0.4  
Weight Required = 514.62 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 205.85 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	310 lbs	310 lbs	310 lbs	310 lbs	431 lbs	431 lbs	431 lbs	431 lbs	530 lbs	530 lbs	530 lbs	530 lbs	-91 lbs	-91 lbs	-91 lbs	-91 lbs
$F_B$	213 lbs	213 lbs	213 lbs	213 lbs	464 lbs	464 lbs	464 lbs	464 lbs	489 lbs	489 lbs	489 lbs	489 lbs	-794 lbs	-794 lbs	-794 lbs	-794 lbs
$F_V$	21 lbs	21 lbs	21 lbs	21 lbs	364 lbs	364 lbs	364 lbs	364 lbs	287 lbs	287 lbs	287 lbs	287 lbs	-412 lbs	-412 lbs	-412 lbs	-412 lbs
$P_{total}$	2518 lbs	2608 lbs	2699 lbs	2789 lbs	2889 lbs	2979 lbs	3070 lbs	3161 lbs	3013 lbs	3103 lbs	3194 lbs	3285 lbs	311 lbs	366 lbs	420 lbs	475 lbs
$M$	220 lbs-ft	220 lbs-ft	220 lbs-ft	220 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	527 lbs-ft	527 lbs-ft	527 lbs-ft	527 lbs-ft	644 lbs-ft	644 lbs-ft	644 lbs-ft	644 lbs-ft
$e$	0.09 ft	0.08 ft	0.08 ft	0.08 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.17 ft	0.17 ft	0.17 ft	0.16 ft	2.07 ft	1.76 ft	1.53 ft	1.36 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	245.8 psf	244.6 psf	243.5 psf	242.4 psf	249.0 psf	247.7 psf	246.4 psf	245.2 psf	259.7 psf	257.8 psf	256.2 psf	254.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	303.4 psf	299.7 psf	296.3 psf	293.1 psf	381.3 psf	374.1 psf	367.6 psf	361.6 psf	397.7 psf	389.8 psf	382.7 psf	376.0 psf	262.0 psf	172.0 psf	144.8 psf	132.9 psf

Maximum Bearing Pressure = 398 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

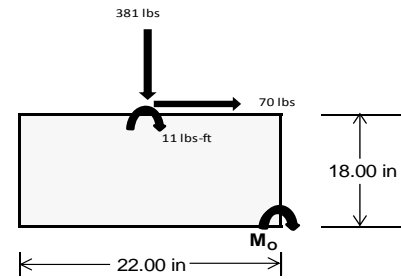
### Overturning Check

$M_o = 233.4$  ft-lbs  
 Resisting Force Required = 254.61 lbs  
 S.F. = 1.67  
 Weight Required = 424.35 lbs  
 Minimum Width = 22 in in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	109 lbs	44 lbs	50 lbs	201 lbs	381 lbs	156 lbs	76 lbs	-35 lbs	18 lbs
$F_v$	11 lbs	93 lbs	11 lbs	8 lbs	70 lbs	8 lbs	11 lbs	93 lbs	11 lbs
$P_{total}$	2578 lbs	2512 lbs	2518 lbs	2551 lbs	2730 lbs	2505 lbs	798 lbs	687 lbs	740 lbs
$M$	31 lbs-ft	154 lbs-ft	32 lbs-ft	22 lbs-ft	116 lbs-ft	25 lbs-ft	31 lbs-ft	154 lbs-ft	32 lbs-ft
$e$	0.01 ft	0.06 ft	0.01 ft	0.01 ft	0.04 ft	0.01 ft	0.04 ft	0.22 ft	0.04 ft
$L/6$	0.31 ft	1.71 ft	1.81 ft	1.82 ft	1.75 ft	1.81 ft	1.76 ft	1.38 ft	1.75 ft
$f_{min}$	270.2 sqft	219.1 sqft	263.4 sqft	270.3 sqft	256.6 sqft	264.5 sqft	76.0 sqft	19.9 sqft	69.5 sqft
$f_{max}$	292.2 psf	329.1 psf	286.0 psf	286.1 psf	339.1 psf	282.1 psf	98.1 psf	129.9 psf	92.0 psf



Maximum Bearing Pressure = 339 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



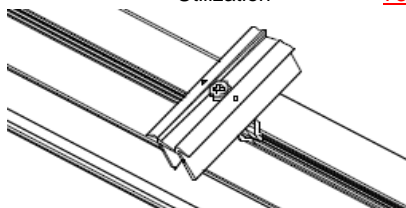
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

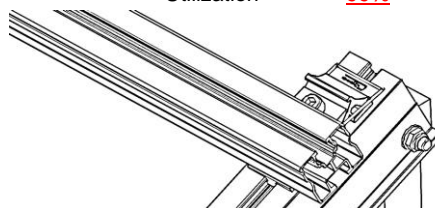
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.882 k
Allowable Uplift =	1.214 k
Utilization =	<u>73%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.105 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

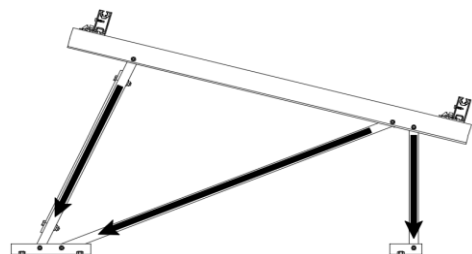
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.798 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>

#### Diagonal Strut

Maximum Axial Load =	0.452 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



#### Rear Strut

Maximum Axial Load =	1.093 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.159 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	30.83 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.617 in
Max Drift, $\Delta_{MAX}$ =	0.047 in
	<u>0.047 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.0 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.276 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &21.4323 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

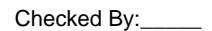
$$P_{\max} = 5.82 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





RISA-3D Version 13.0.0    \...\...\PVMMini 60 Cell 1V 25° 130mph 30psf 3.5ft 7-05.r    Page 21



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	208.816	2	.036	2	.068	1	0	10	0	4	0	15
30		min	-367.684	3	-.031	3	-.536	5	0	4	0	3	0	6
31	16	max	208.933	2	0	2	.068	1	0	10	0	4	0	15
32		min	-367.597	3	-.058	3	-.642	5	0	4	0	3	0	6
33	17	max	209.049	2	-.022	15	.068	1	0	10	0	4	0	15
34		min	-367.51	3	-.089	4	-.747	5	0	4	0	3	0	6
35	18	max	209.166	2	-.033	15	.068	1	0	10	0	9	0	15
36		min	-367.423	3	-.135	4	-.853	5	0	4	0	3	0	6
37	19	max	209.282	2	-.044	15	.068	1	0	10	0	9	0	15
38		min	-367.335	3	-.181	4	-.958	5	0	4	0	3	0	6
39	M3	1	max	151.341	2	1.777	.012	10	0	5	0	4	0	6
40		min	-136.832	3	.417	15	-1.327	4	0	1	0	10	0	15
41	2	max	151.273	2	1.6	6	.012	10	0	5	0	1	0	2
42		min	-136.884	3	.375	15	-1.194	4	0	1	0	10	0	12
43	3	max	151.204	2	1.422	6	.012	10	0	5	0	1	0	2
44		min	-136.935	3	.333	15	-1.06	4	0	1	0	5	0	3
45	4	max	151.135	2	1.245	6	.012	10	0	5	0	1	0	15
46		min	-136.987	3	.292	15	-.926	4	0	1	0	5	0	4
47	5	max	151.067	2	1.068	6	.012	10	0	5	0	1	0	15
48		min	-137.038	3	.25	15	-.793	4	0	1	0	5	0	4
49	6	max	150.998	2	.891	6	.012	10	0	5	0	1	0	15
50		min	-137.089	3	.208	15	-.659	4	0	1	0	5	0	4
51	7	max	150.93	2	.714	6	.012	10	0	5	0	1	0	15
52		min	-137.141	3	.167	15	-.525	4	0	1	0	5	0	4
53	8	max	150.861	2	.536	6	.012	10	0	5	0	1	0	15
54		min	-137.192	3	.125	15	-.392	4	0	1	0	5	-.001	4
55	9	max	150.792	2	.359	6	.012	10	0	5	0	1	0	15
56		min	-137.244	3	.083	15	-.258	4	0	1	0	5	-.001	4
57	10	max	150.724	2	.182	6	.012	10	0	5	0	1	0	15
58		min	-137.295	3	.042	15	-.125	4	0	1	0	5	-.001	4
59	11	max	150.655	2	.031	2	.034	5	0	5	0	1	0	15
60		min	-137.347	3	-.023	3	-.102	1	0	1	0	5	-.001	4
61	12	max	150.587	2	-.042	15	.168	5	0	5	0	1	0	15
62		min	-137.398	3	-.173	4	-.102	1	0	1	0	5	-.001	4
63	13	max	150.518	2	-.083	15	.301	5	0	5	0	1	0	15
64		min	-137.45	3	-.35	4	-.102	1	0	1	0	5	-.001	4
65	14	max	150.449	2	-.125	15	.435	5	0	5	0	9	0	15
66		min	-137.501	3	-.527	4	-.102	1	0	1	0	5	-.001	4
67	15	max	150.381	2	-.167	15	.569	5	0	5	0	9	0	15
68		min	-137.553	3	-.704	4	-.102	1	0	1	0	5	0	4
69	16	max	150.312	2	-.208	15	.702	5	0	5	0	10	0	15
70		min	-137.604	3	-.881	4	-.102	1	0	1	0	4	0	4
71	17	max	150.243	2	-.25	15	.836	5	0	5	0	10	0	15
72		min	-137.655	3	-1.059	4	-.102	1	0	1	0	4	0	4
73	18	max	150.175	2	-.292	15	.969	5	0	5	0	10	0	15
74		min	-137.707	3	-1.236	4	-.102	1	0	1	0	4	0	4
75	19	max	150.106	2	-.333	15	1.103	5	0	5	0	5	0	1
76		min	-137.758	3	-1.413	4	-.102	1	0	1	0	1	0	1
77	M4	1	max	269.203	1	0	.058	10	0	1	0	5	0	1
78		min	-33.224	3	0	1	-13.31	4	0	1	0	2	0	1
79	2	max	269.267	1	0	1	.058	10	0	1	0	10	0	1
80		min	-33.175	3	0	1	-13.366	4	0	1	-.001	4	0	1
81	3	max	269.332	1	0	1	.058	10	0	1	0	10	0	1
82		min	-33.126	3	0	1	-13.422	4	0	1	-.002	4	0	1
83	4	max	269.397	1	0	1	.058	10	0	1	0	10	0	1
84		min	-33.078	3	0	1	-13.478	4	0	1	-.004	4	0	1
85	5	max	269.462	1	0	1	.058	10	0	1	0	10	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-33.029	3	0	1	-13.534	4	0	1	-.005	4	0	1
87	6	max	269.526	1	0	1	.058	10	0	1	0	10	0	1
88		min	-32.981	3	0	1	-13.59	4	0	1	-.006	4	0	1
89	7	max	269.591	1	0	1	.058	10	0	1	0	10	0	1
90		min	-32.932	3	0	1	-13.646	4	0	1	-.007	4	0	1
91	8	max	269.656	1	0	1	.058	10	0	1	0	10	0	1
92		min	-32.884	3	0	1	-13.703	4	0	1	-.008	4	0	1
93	9	max	269.72	1	0	1	.058	10	0	1	0	10	0	1
94		min	-32.835	3	0	1	-13.759	4	0	1	-.01	4	0	1
95	10	max	269.785	1	0	1	.058	10	0	1	0	10	0	1
96		min	-32.787	3	0	1	-13.815	4	0	1	-.011	4	0	1
97	11	max	269.85	1	0	1	.058	10	0	1	0	10	0	1
98		min	-32.738	3	0	1	-13.871	4	0	1	-.012	4	0	1
99	12	max	269.914	1	0	1	.058	10	0	1	0	10	0	1
100		min	-32.69	3	0	1	-13.927	4	0	1	-.013	4	0	1
101	13	max	269.979	1	0	1	.058	10	0	1	0	10	0	1
102		min	-32.641	3	0	1	-13.983	4	0	1	-.015	4	0	1
103	14	max	270.044	1	0	1	.058	10	0	1	0	10	0	1
104		min	-32.593	3	0	1	-14.039	4	0	1	-.016	4	0	1
105	15	max	270.109	1	0	1	.058	10	0	1	0	10	0	1
106		min	-32.544	3	0	1	-14.095	4	0	1	-.017	4	0	1
107	16	max	270.173	1	0	1	.058	10	0	1	0	10	0	1
108		min	-32.496	3	0	1	-14.151	4	0	1	-.018	4	0	1
109	17	max	270.238	1	0	1	.058	10	0	1	0	10	0	1
110		min	-32.447	3	0	1	-14.207	4	0	1	-.02	4	0	1
111	18	max	270.303	1	0	1	.058	10	0	1	0	10	0	1
112		min	-32.399	3	0	1	-14.263	4	0	1	-.021	4	0	1
113	19	max	270.367	1	0	1	.058	10	0	1	0	10	0	1
114		min	-32.35	3	0	1	-14.319	4	0	1	-.022	4	0	1
115	M6	1	max	637.027	2	.628	.928	4	0	3	0	3	0	1
116		min	-1092.756	3	.141	15	-.313	3	0	5	0	1	0	1
117	2	max	637.144	2	.582	6	.822	4	0	3	0	3	0	15
118		min	-1092.669	3	.13	15	-.313	3	0	5	0	1	0	6
119	3	max	637.26	2	.536	6	.717	4	0	3	0	4	0	15
120		min	-1092.582	3	.119	15	-.313	3	0	5	0	1	0	6
121	4	max	637.377	2	.491	6	.611	4	0	3	0	4	0	15
122		min	-1092.494	3	.108	15	-.313	3	0	5	0	1	0	6
123	5	max	637.493	2	.449	2	.506	4	0	3	0	4	0	15
124		min	-1092.407	3	.098	15	-.313	3	0	5	0	1	0	6
125	6	max	637.609	2	.414	2	.4	4	0	3	0	4	0	15
126		min	-1092.32	3	.087	15	-.313	3	0	5	0	1	0	6
127	7	max	637.726	2	.378	2	.295	4	0	3	0	4	0	15
128		min	-1092.232	3	.076	15	-.313	3	0	5	0	3	0	6
129	8	max	637.842	2	.343	2	.19	4	0	3	0	4	0	15
130		min	-1092.145	3	.065	12	-.313	3	0	5	0	3	0	6
131	9	max	637.959	2	.307	2	.084	4	0	3	0	4	0	15
132		min	-1092.058	3	.047	12	-.313	3	0	5	0	3	0	2
133	10	max	638.075	2	.272	2	.013	9	0	3	0	4	0	15
134		min	-1091.971	3	.03	12	-.313	3	0	5	0	3	0	2
135	11	max	638.191	2	.236	2	.013	9	0	3	0	4	0	15
136		min	-1091.883	3	.012	3	-.313	3	0	5	0	3	0	2
137	12	max	638.308	2	.2	2	.013	9	0	3	0	4	0	15
138		min	-1091.796	3	-.015	3	-.313	3	0	5	0	3	0	2
139	13	max	638.424	2	.165	2	.013	9	0	3	0	4	0	15
140		min	-1091.709	3	-.042	3	-.343	5	0	5	0	3	0	2
141	14	max	638.541	2	.129	2	.013	9	0	3	0	4	0	15
142		min	-1091.621	3	-.069	3	-.449	5	0	5	0	3	0	2





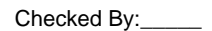
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

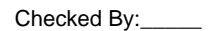
Dec 11, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	638.657	2	.094	2	.013	9	0	3	0	4	0	12
144			min	-1091.534	3	-.095	3	-.554	5	0	5	0	3	0	2
145		16	max	638.773	2	.058	2	.013	9	0	3	0	4	0	12
146			min	-1091.447	3	-.122	3	-.66	5	0	5	0	3	0	2
147		17	max	638.89	2	.022	2	.013	9	0	3	0	4	0	12
148			min	-1091.359	3	-.149	3	-.765	5	0	5	0	3	0	2
149		18	max	639.006	2	-.013	2	.013	9	0	3	0	4	0	12
150			min	-1091.272	3	-.175	3	-.871	5	0	5	0	3	0	2
151		19	max	639.123	2	-.049	2	.013	9	0	3	0	9	0	12
152			min	-1091.185	3	-.202	3	-.976	5	0	5	0	3	0	2
153	M7	1	max	451.867	2	1.792	4	.027	3	0	9	0	4	0	2
154			min	-353.163	3	.427	15	-1.347	4	0	3	0	3	0	12
155		2	max	451.798	2	1.615	4	.027	3	0	9	0	4	0	2
156			min	-353.215	3	.385	15	-1.213	4	0	3	0	3	0	3
157		3	max	451.73	2	1.437	4	.027	3	0	9	0	9	0	2
158			min	-353.266	3	.343	15	-1.08	4	0	3	0	3	0	3
159		4	max	451.661	2	1.26	4	.027	3	0	9	0	9	0	2
160			min	-353.318	3	.302	15	-.946	4	0	3	0	3	0	3
161		5	max	451.593	2	1.083	4	.027	3	0	9	0	9	0	15
162			min	-353.369	3	.26	15	-.812	4	0	3	0	5	0	3
163		6	max	451.524	2	.906	4	.027	3	0	9	0	9	0	15
164			min	-353.421	3	.218	15	-.679	4	0	3	0	5	0	6
165		7	max	451.455	2	.729	4	.027	3	0	9	0	9	0	15
166			min	-353.472	3	.177	15	-.545	4	0	3	0	5	0	6
167		8	max	451.387	2	.551	4	.027	3	0	9	0	9	0	15
168			min	-353.524	3	.135	15	-.411	4	0	3	0	5	0	6
169		9	max	451.318	2	.374	4	.027	3	0	9	0	9	0	15
170			min	-353.575	3	.093	15	-.278	4	0	3	0	5	-.001	6
171		10	max	451.249	2	.214	2	.027	3	0	9	0	9	0	15
172			min	-353.627	3	.027	12	-.144	4	0	3	0	5	-.001	6
173		11	max	451.181	2	.076	2	.027	3	0	9	0	9	0	15
174			min	-353.678	3	-.067	3	-.011	4	0	3	0	5	-.001	6
175		12	max	451.112	2	-.032	15	.125	5	0	9	0	9	0	15
176			min	-353.729	3	-.17	3	-.003	9	0	3	0	5	-.001	6
177		13	max	451.044	2	-.073	15	.258	5	0	9	0	9	0	15
178			min	-353.781	3	-.335	6	-.003	9	0	3	0	5	-.001	6
179		14	max	450.975	2	-.115	15	.392	5	0	9	0	9	0	15
180			min	-353.832	3	-.512	6	-.003	9	0	3	0	5	-.001	6
181		15	max	450.906	2	-.157	15	.525	5	0	9	0	9	0	15
182			min	-353.884	3	-.689	6	-.003	9	0	3	0	5	0	6
183		16	max	450.838	2	-.198	15	.659	5	0	9	0	9	0	15
184			min	-353.935	3	-.867	6	-.003	9	0	3	0	5	0	6
185		17	max	450.769	2	-.24	15	.793	5	0	9	0	9	0	15
186			min	-353.987	3	-1.044	6	-.003	9	0	3	0	5	0	6
187		18	max	450.701	2	-.282	15	.926	5	0	9	0	9	0	15
188			min	-354.038	3	-1.221	6	-.003	9	0	3	0	3	0	6
189		19	max	450.632	2	-.323	15	1.06	5	0	9	0	9	0	1
190			min	-354.09	3	-1.398	6	-.003	9	0	3	0	3	0	1
191	M8	1	max	796.352	2	0	1	.076	9	0	1	0	4	0	1
192			min	-148.887	3	0	1	-13.594	4	0	1	0	3	0	1
193		2	max	796.417	2	0	1	.076	9	0	1	0	9	0	1
194			min	-148.838	3	0	1	-13.65	4	0	1	-.001	4	0	1
195		3	max	796.482	2	0	1	.076	9	0	1	0	9	0	1
196			min	-148.79	3	0	1	-13.707	4	0	1	-.002	4	0	1
197		4	max	796.546	2	0	1	.076	9	0	1	0	9	0	1
198			min	-148.741	3	0	1	-13.763	4	0	1	-.004	4	0	1
199		5	max	796.611	2	0	1	.076	9	0	1	0	9	0	1







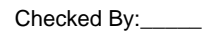
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

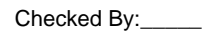
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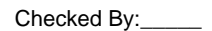
### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-32.403	3	0	1	-12.742	5	0	1	-.005	5	0	1
315	6	max	269.83	1	0	1	.442	3	0	1	0	1	0	1
316		min	-32.355	3	0	1	-12.798	5	0	1	-.006	5	0	1
317	7	max	269.894	1	0	1	.442	3	0	1	0	1	0	1
318		min	-32.306	3	0	1	-12.854	5	0	1	-.007	5	0	1
319	8	max	269.959	1	0	1	.442	3	0	1	0	1	0	1
320		min	-32.257	3	0	1	-12.91	5	0	1	-.008	5	0	1
321	9	max	270.024	1	0	1	.442	3	0	1	0	3	0	1
322		min	-32.209	3	0	1	-12.967	5	0	1	-.009	5	0	1
323	10	max	270.088	1	0	1	.442	3	0	1	0	3	0	1
324		min	-32.16	3	0	1	-13.023	5	0	1	-.01	5	0	1
325	11	max	270.153	1	0	1	.442	3	0	1	0	3	0	1
326		min	-32.112	3	0	1	-13.079	5	0	1	-.011	5	0	1
327	12	max	270.218	1	0	1	.442	3	0	1	0	3	0	1
328		min	-32.063	3	0	1	-13.135	5	0	1	-.013	5	0	1
329	13	max	270.282	1	0	1	.442	3	0	1	0	3	0	1
330		min	-32.015	3	0	1	-13.191	5	0	1	-.014	5	0	1
331	14	max	270.347	1	0	1	.442	3	0	1	0	3	0	1
332		min	-31.966	3	0	1	-13.247	5	0	1	-.015	5	0	1
333	15	max	270.412	1	0	1	.442	3	0	1	0	3	0	1
334		min	-31.918	3	0	1	-13.303	5	0	1	-.016	5	0	1
335	16	max	270.477	1	0	1	.442	3	0	1	0	3	0	1
336		min	-31.869	3	0	1	-13.359	5	0	1	-.017	5	0	1
337	17	max	270.541	1	0	1	.442	3	0	1	0	3	0	1
338		min	-31.821	3	0	1	-13.415	5	0	1	-.019	5	0	1
339	18	max	270.606	1	0	1	.442	3	0	1	0	3	0	1
340		min	-31.772	3	0	1	-13.471	5	0	1	-.02	5	0	1
341	19	max	270.671	1	0	1	.442	3	0	1	0	3	0	1
342		min	-31.724	3	0	1	-13.527	5	0	1	-.021	5	0	1
343	M1	1	max	60.556	1	348.367	3	1.373	10	0	.025	4	0	2
344		min	2.95	10	-225.463	2	-14.518	4	0	3	-.003	10	0	3
345	2	max	60.674	1	348.177	3	1.373	10	0	2	.022	4	.049	2
346		min	3.048	10	-225.716	2	-14.276	4	0	3	-.002	10	-.076	3
347	3	max	63.397	3	4.53	4	1.368	10	0	5	.019	4	.097	2
348		min	-12.058	10	-20.668	2	-13.071	4	0	1	-.002	10	-.15	3
349	4	max	63.486	3	4.205	4	1.368	10	0	5	.016	4	.102	2
350		min	-11.959	10	-20.921	2	-12.829	4	0	1	-.002	10	-.146	3
351	5	max	63.574	3	3.881	4	1.368	10	0	5	.013	4	.106	2
352		min	-11.861	10	-21.174	2	-12.587	4	0	1	-.001	10	-.142	3
353	6	max	63.663	3	3.556	4	1.368	10	0	5	.01	4	.111	2
354		min	-11.763	10	-21.427	2	-12.345	4	0	1	-.001	10	-.138	3
355	7	max	63.751	3	3.294	14	1.368	10	0	5	.008	4	.116	2
356		min	-11.664	10	-21.68	2	-12.103	4	0	1	0	10	-.135	3
357	8	max	63.84	3	3.045	14	1.368	10	0	5	.005	4	.12	2
358		min	-11.566	10	-21.933	2	-11.861	4	0	1	0	10	-.131	3
359	9	max	63.928	3	2.797	14	1.368	10	0	5	.003	3	.125	2
360		min	-11.468	10	-22.186	2	-11.619	4	0	1	0	10	-.127	3
361	10	max	64.017	3	2.548	14	1.368	10	0	5	.002	3	.13	2
362		min	-11.369	10	-22.439	2	-11.377	4	0	1	0	10	-.123	3
363	11	max	64.106	3	2.299	14	1.368	10	0	5	0	3	.135	2
364		min	-11.271	10	-22.692	2	-11.329	1	0	1	-.002	1	-.119	3
365	12	max	64.194	3	2.051	14	1.368	10	0	5	0	10	.14	2
366		min	-11.173	10	-22.946	2	-11.329	1	0	1	-.005	1	-.114	3
367	13	max	64.283	3	1.802	14	1.368	10	0	5	0	10	.145	2
368		min	-11.074	10	-23.199	2	-11.329	1	0	1	-.007	1	-.11	3
369	14	max	64.371	3	1.553	14	1.368	10	0	5	.001	10	.15	2
370		min	-10.976	10	-23.452	2	-11.329	1	0	1	-.01	1	-.106	3











Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-117.464	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.325	3	.172	3	0	1	0	1	0	1
544		min	-117.529	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.244	3	.172	3	0	1	0	3	0	1
546		min	-117.594	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.162	3	.172	3	0	1	0	3	0	1
548		min	-117.66	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.081	3	.172	3	0	1	0	3	0	1
550		min	-117.725	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.172	3	0	1	0	3	0	1
552		min	-117.79	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.172	3	0	1	0	3	0	1
554		min	-117.855	3	-.081	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.172	3	0	1	0	3	0	1
556		min	-117.92	3	-.162	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.172	3	0	1	0	3	0	1
558		min	-117.986	3	-.244	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.172	3	0	1	0	3	0	1
560		min	-118.051	3	-.325	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.172	3	0	1	0	3	0	1
562		min	-118.116	3	-.406	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.172	3	0	1	0	3	0	1
564		min	-118.181	3	-.487	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.172	3	0	1	0	3	0	1
566		min	-118.246	3	-.569	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.172	3	0	1	0	3	0	1
568		min	-118.312	3	-.65	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.172	3	0	1	0	3	0	1
570		min	-118.377	3	-.731	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.908	.298	4	0	3	0	3	0	1
572		min	-159.044	4	0	1	-.07	3	0	4	0	4	0	1
573	2	max	0	1	1.696	4	.269	4	0	3	0	3	0	1
574		min	-159.023	4	0	1	-.07	3	0	4	0	4	0	4
575	3	max	0	1	1.484	4	.239	4	0	3	0	3	0	1
576		min	-159.003	4	0	1	-.07	3	0	4	0	4	0	4
577	4	max	0	1	1.272	4	.21	4	0	3	0	3	0	1
578		min	-158.982	4	0	1	-.07	3	0	4	0	4	-.001	4
579	5	max	0	1	1.06	4	.18	4	0	3	0	3	0	1
580		min	-158.962	4	0	1	-.07	3	0	4	0	9	-.002	4
581	6	max	0	1	.848	4	.151	4	0	3	0	3	0	1
582		min	-158.941	4	0	1	-.07	3	0	4	0	9	-.002	4
583	7	max	0	1	.636	4	.121	4	0	3	0	3	0	1
584		min	-158.92	4	0	1	-.07	3	0	4	0	9	-.002	4
585	8	max	0	1	.424	4	.092	4	0	3	0	5	0	1
586		min	-158.9	4	0	1	-.07	3	0	4	0	9	-.002	4
587	9	max	0	1	.212	4	.062	4	0	3	0	5	0	1
588		min	-158.879	4	0	1	-.07	3	0	4	0	9	-.002	4
589	10	max	0	1	0	1	.033	4	0	3	0	5	0	1
590		min	-158.859	4	0	1	-.07	3	0	4	0	9	-.002	4
591	11	max	0	1	0	1	.015	9	0	3	0	5	0	1
592		min	-158.838	4	-.212	4	-.07	3	0	4	0	9	-.002	4
593	12	max	0	1	0	1	.015	9	0	3	0	5	0	1
594		min	-158.818	4	-.424	4	-.07	3	0	4	0	9	-.002	4
595	13	max	0	1	0	1	.015	9	0	3	0	5	0	1
596		min	-158.797	4	-.636	4	-.07	3	0	4	0	3	-.002	4
597	14	max	0	1	0	1	.015	9	0	3	0	5	0	1
598		min	-158.777	4	-.848	4	-.089	5	0	4	0	3	-.002	4





Company : Schletter, Inc.  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.019	9	0	1	.015	9	0	3	0	5	0	1
600		min	-158.756	4	-1.06	4	-.118	5	0	4	0	3	-.002	4
601	16	max	.092	9	0	1	.015	9	0	3	0	5	0	1
602		min	-158.794	5	-1.272	4	-.148	5	0	4	0	3	-.001	4
603	17	max	.164	9	0	1	.015	9	0	3	0	9	0	1
604		min	-158.844	5	-1.484	4	-.177	5	0	4	0	3	0	4
605	18	max	.237	9	0	1	.015	9	0	3	0	9	0	1
606		min	-158.894	5	-1.696	4	-.207	5	0	4	0	3	0	4
607	19	max	.309	9	0	1	.015	9	0	3	0	9	0	1
608		min	-158.945	5	-1.908	4	-.236	5	0	4	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.008	2	.001	9	7.583e-4	5	NC	3	NC	1
2			min	-.003	3	-.008	3	-.008	5	-1.989e-4	3	4439.668	2	NC	1
3		2	max	.002	2	.008	2	.001	9	7.783e-4	5	NC	3	NC	1
4			min	-.003	3	-.008	3	-.008	5	-1.884e-4	3	4834.623	2	NC	1
5		3	max	.002	2	.007	2	.001	9	7.983e-4	5	NC	3	NC	1
6			min	-.003	3	-.007	3	-.008	5	-1.779e-4	3	5302.48	2	NC	1
7		4	max	.002	2	.006	2	.001	9	8.182e-4	5	NC	1	NC	1
8			min	-.003	3	-.007	3	-.008	5	-1.675e-4	3	5860.574	2	NC	1
9		5	max	.001	2	.006	2	.001	9	8.382e-4	5	NC	1	NC	1
10			min	-.003	3	-.007	3	-.007	5	-1.57e-4	3	6531.938	2	NC	1
11		6	max	.001	2	.005	2	0	9	8.581e-4	5	NC	1	NC	1
12			min	-.002	3	-.006	3	-.007	5	-1.465e-4	3	7347.677	2	NC	1
13		7	max	.001	2	.004	2	0	9	8.781e-4	5	NC	1	NC	1
14			min	-.002	3	-.006	3	-.007	5	-1.36e-4	3	8350.604	2	NC	1
15		8	max	.001	2	.004	2	0	9	8.981e-4	5	NC	1	NC	1
16			min	-.002	3	-.006	3	-.006	5	-1.256e-4	3	9600.97	2	NC	1
17		9	max	.001	2	.003	2	0	9	9.18e-4	5	NC	1	NC	1
18			min	-.002	3	-.005	3	-.006	5	-1.151e-4	3	NC	1	NC	1
19		10	max	0	2	.003	2	0	9	9.38e-4	5	NC	1	NC	1
20		min	-.002	3	-.005	3	-.006	5	-1.046e-4	3	NC	1	NC	1	
21	11	max	0	2	.002	2	0	9	9.58e-4	5	NC	1	NC	1	
22		min	-.001	3	-.004	3	-.005	5	-9.58e-5	1	NC	1	NC	1	
23	12	max	0	2	.002	2	0	9	9.779e-4	5	NC	1	NC	1	
24		min	-.001	3	-.004	3	-.005	5	-8.745e-5	1	NC	1	NC	1	
25	13	max	0	2	.001	2	0	9	9.979e-4	5	NC	1	NC	1	
26		min	-.001	3	-.003	3	-.004	5	-7.91e-5	1	NC	1	NC	1	
27	14	max	0	2	.001	2	0	9	1.018e-3	5	NC	1	NC	1	
28		min	0	3	-.003	3	-.003	5	-7.075e-5	1	NC	1	NC	1	
29	15	max	0	2	0	2	0	9	1.038e-3	5	NC	1	NC	1	
30		min	0	3	-.002	3	-.003	5	-6.24e-5	1	NC	1	NC	1	
31	16	max	0	2	0	2	0	9	1.058e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.002	5	-5.406e-5	1	NC	1	NC	1	
33	17	max	0	2	0	2	0	9	1.078e-3	5	NC	1	NC	1	
34		min	0	3	-.001	3	-.001	5	-4.571e-5	1	NC	1	NC	1	
35	18	max	0	2	0	2	0	9	1.098e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-3.736e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.118e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-2.917e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.36e-5	9	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.198e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	1.971e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	9	-5.219e-4	5	NC	1	NC	1



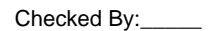
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.005	5	2.583e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.24e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.008	5	3.196e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	9	-5.261e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.011	4	3.809e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.282e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.013	4	4.421e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.304e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.016	4	5.034e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-5.325e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.019	4	5.647e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.346e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.021	4	6.259e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-5.367e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.023	4	6.872e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.388e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.026	4	7.484e-5	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-5.41e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.028	4	8.097e-5	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	10	-5.431e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.03	4	8.71e-5	1	NC	1	NC	1
64			min	-.001	2	-.007	3	0	10	-5.452e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.032	4	9.322e-5	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	10	-5.473e-4	5	NC	1	NC	1
67		15	max	.001	3	.005	2	.034	4	9.935e-5	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	-5.494e-4	5	9557.524	2	NC	1
69		16	max	.001	3	.006	2	.036	4	1.055e-4	1	NC	1	NC	1
70			min	-.001	2	-.008	3	0	10	-5.516e-4	5	8070.894	2	NC	1
71		17	max	.001	3	.007	2	.037	4	1.116e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-5.537e-4	5	6925.612	2	NC	1
73		18	max	.001	3	.008	2	.039	4	1.177e-4	1	NC	3	NC	1
74			min	-.002	2	-.008	3	0	10	-5.558e-4	5	6032.977	2	NC	1
75		19	max	.002	3	.009	2	.041	4	1.239e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-5.579e-4	5	5330.867	2	NC	1
77	M4	1	max	.001	1	.009	2	0	10	2.392e-3	5	NC	1	NC	1
78			min	0	3	-.008	3	-.043	4	-1.331e-4	1	NC	1	447.418	4
79		2	max	.001	1	.009	2	0	10	2.392e-3	5	NC	1	NC	1
80			min	0	3	-.008	3	-.04	4	-1.331e-4	1	NC	1	487.671	4
81		3	max	.001	1	.008	2	0	10	2.392e-3	5	NC	1	NC	1
82			min	0	3	-.007	3	-.036	4	-1.331e-4	1	NC	1	535.571	4
83		4	max	.001	1	.008	2	0	10	2.392e-3	5	NC	1	NC	1
84			min	0	3	-.007	3	-.033	4	-1.331e-4	1	NC	1	593.131	4
85		5	max	0	1	.007	2	0	10	2.392e-3	5	NC	1	NC	1
86			min	0	3	-.006	3	-.029	4	-1.331e-4	1	NC	1	663.094	4
87		6	max	0	1	.007	2	0	10	2.392e-3	5	NC	1	NC	1
88			min	0	3	-.006	3	-.026	4	-1.331e-4	1	NC	1	749.275	4
89		7	max	0	1	.006	2	0	10	2.392e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.023	4	-1.331e-4	1	NC	1	857.105	4
91		8	max	0	1	.006	2	0	10	2.392e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.019	4	-1.331e-4	1	NC	1	994.536	4
93		9	max	0	1	.005	2	0	10	2.392e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.016	4	-1.331e-4	1	NC	1	1173.598	4
95		10	max	0	1	.005	2	0	10	2.392e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.014	4	-1.331e-4	1	NC	1	1413.195	4
97		11	max	0	1	.004	2	0	10	2.392e-3	5	NC	1	NC	1
98			min	0	3	-.004	3	-.011	4	-1.331e-4	1	NC	1	1744.462	4
99		12	max	0	1	.004	2	0	10	2.392e-3	5	NC	1	NC	1





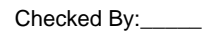
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	4.454e-6	9	NC	1	NC	1
158			min	0	2	-.003	3	0	9	-5.271e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.008	4	3.843e-6	9	NC	1	NC	1
160			min	0	2	-.005	3	0	9	-5.197e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.011	4	3.233e-6	9	NC	1	NC	1
162			min	-.001	2	-.007	3	0	9	-5.123e-4	4	9905.782	2	NC	1
163		6	max	.001	3	.006	2	.014	4	1.925e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	9	-5.05e-4	4	7938.613	2	NC	1
165		7	max	.001	3	.007	2	.017	4	4.392e-5	3	NC	1	NC	1
166			min	-.002	2	-.01	3	0	9	-4.976e-4	4	6588.65	2	NC	1
167		8	max	.002	3	.008	2	.019	4	6.859e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	9	-4.902e-4	4	5595.727	2	NC	1
169		9	max	.002	3	.01	2	.022	4	9.325e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	9	-4.828e-4	4	4830.346	2	NC	1
171		10	max	.002	3	.011	2	.024	4	1.179e-4	3	NC	3	NC	1
172			min	-.003	2	-.014	3	0	9	-4.755e-4	4	4220.754	2	NC	1
173		11	max	.002	3	.012	2	.027	4	1.426e-4	3	NC	3	NC	1
174			min	-.003	2	-.015	3	0	9	-4.681e-4	4	3723.828	2	NC	1
175		12	max	.002	3	.014	2	.029	4	1.673e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	0	9	-4.607e-4	4	3311.894	2	NC	1
177		13	max	.003	3	.016	2	.031	4	1.919e-4	3	NC	3	NC	1
178			min	-.003	2	-.018	3	0	9	-4.533e-4	4	2966.203	2	NC	1
179		14	max	.003	3	.017	2	.033	4	2.166e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	0	9	-4.459e-4	4	2673.458	2	NC	1
181		15	max	.003	3	.019	2	.035	4	2.413e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	0	9	-4.386e-4	4	2423.869	2	NC	1
183		16	max	.003	3	.021	2	.037	4	2.659e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	0	9	-4.312e-4	4	2210	2	NC	1
185		17	max	.004	3	.023	2	.039	4	2.906e-4	3	NC	3	NC	1
186			min	-.005	2	-.021	3	0	9	-4.238e-4	4	2026.069	2	NC	1
187		18	max	.004	3	.025	2	.04	4	3.153e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	0	9	-4.164e-4	4	1867.497	2	NC	1
189		19	max	.004	3	.027	2	.042	4	3.399e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	0	9	-4.09e-4	4	1730.61	2	NC	1
191	M8	1	max	.004	2	.028	2	0	9	2.253e-3	4	NC	1	NC	1
192			min	0	3	-.023	3	-.044	4	-2.501e-4	3	NC	1	438.328	4
193		2	max	.004	2	.027	2	0	9	2.253e-3	4	NC	1	NC	1
194			min	0	3	-.022	3	-.04	4	-2.501e-4	3	NC	1	477.766	4
195		3	max	.003	2	.025	2	0	9	2.253e-3	4	NC	1	NC	1
196			min	0	3	-.02	3	-.037	4	-2.501e-4	3	NC	1	524.696	4
197		4	max	.003	2	.024	2	0	9	2.253e-3	4	NC	1	NC	1
198			min	0	3	-.019	3	-.033	4	-2.501e-4	3	NC	1	581.091	4
199		5	max	.003	2	.022	2	0	9	2.253e-3	4	NC	1	NC	1
200			min	0	3	-.018	3	-.03	4	-2.501e-4	3	NC	1	649.639	4
201		6	max	.003	2	.02	2	0	9	2.253e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.026	4	-2.501e-4	3	NC	1	734.076	4
203		7	max	.003	2	.019	2	0	9	2.253e-3	4	NC	1	NC	1
204			min	0	3	-.015	3	-.023	4	-2.501e-4	3	NC	1	839.726	4
205		8	max	.002	2	.017	2	0	9	2.253e-3	4	NC	1	NC	1
206			min	0	3	-.014	3	-.02	4	-2.501e-4	3	NC	1	974.379	4
207		9	max	.002	2	.016	2	0	9	2.253e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.017	4	-2.501e-4	3	NC	1	1149.822	4
209		10	max	.002	2	.014	2	0	9	2.253e-3	4	NC	1	NC	1
210			min	0	3	-.011	3	-.014	4	-2.501e-4	3	NC	1	1384.579	4
211		11	max	.002	2	.013	2	0	9	2.253e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.011	4	-2.501e-4	3	NC	1	1709.156	4
213		12	max	.001	2	.011	2	0	9	2.253e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.974e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	-.001	3	-4.991e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.007	4	3.563e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.332e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.009	4	1.153e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.672e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.011	4	5.318e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-6.012e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.014	4	6.065e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-6.353e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.016	5	6.813e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.003	3	-6.693e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.018	5	7.56e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-7.033e-4	4	NC	1	NC	1
285		10	max	0	3	.002	2	.02	5	8.307e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-7.374e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.022	5	9.055e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-7.714e-4	4	NC	1	NC	1
289		12	max	0	3	.003	2	.024	5	9.802e-6	10	NC	1	NC	1
290			min	-.001	2	-.007	3	-.003	3	-8.054e-4	4	NC	1	NC	1
291		13	max	.001	3	.003	2	.026	5	1.055e-5	10	NC	1	NC	1
292			min	-.001	2	-.007	3	-.003	3	-8.394e-4	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.028	5	1.13e-5	10	NC	1	NC	1
294			min	-.001	2	-.008	3	-.003	3	-8.735e-4	4	NC	1	NC	1
295		15	max	.001	3	.005	2	.03	5	1.204e-5	10	NC	1	NC	1
296			min	-.001	2	-.008	3	-.003	3	-9.075e-4	4	9569.597	2	NC	1
297		16	max	.001	3	.006	2	.032	5	1.279e-5	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.003	3	-9.415e-4	4	8080.032	2	NC	1
299		17	max	.001	3	.007	2	.034	5	1.354e-5	10	NC	1	NC	1
300			min	-.002	2	-.008	3	-.003	3	-9.756e-4	4	6932.728	2	NC	1
301		18	max	.001	3	.008	2	.035	5	1.429e-5	10	NC	3	NC	1
302			min	-.002	2	-.008	3	-.003	3	-1.01e-3	4	6038.668	2	NC	1
303		19	max	.002	3	.009	2	.037	5	1.503e-5	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.002	3	-1.044e-3	4	5335.538	2	NC	1
305	M12	1	max	.001	1	.009	2	.001	3	2.8e-3	4	NC	1	NC	1
306			min	0	3	-.008	3	-.041	5	-1.647e-5	10	NC	1	474.996	5
307		2	max	.001	1	.009	2	.001	3	2.8e-3	4	NC	1	NC	1
308			min	0	3	-.008	3	-.037	5	-1.647e-5	10	NC	1	517.718	5
309		3	max	.001	1	.008	2	.001	3	2.8e-3	4	NC	1	NC	1
310			min	0	3	-.007	3	-.034	5	-1.647e-5	10	NC	1	568.555	5
311		4	max	.001	1	.008	2	.001	3	2.8e-3	4	NC	1	NC	1
312			min	0	3	-.007	3	-.031	5	-1.647e-5	10	NC	1	629.643	5
313		5	max	.001	1	.007	2	0	3	2.8e-3	4	NC	1	NC	1
314			min	0	3	-.006	3	-.027	5	-1.647e-5	10	NC	1	703.893	5
315		6	max	0	1	.007	2	0	3	2.8e-3	4	NC	1	NC	1
316			min	0	3	-.006	3	-.024	5	-1.647e-5	10	NC	1	795.352	5
317		7	max	0	1	.006	2	0	3	2.8e-3	4	NC	1	NC	1
318			min	0	3	-.005	3	-.021	5	-1.647e-5	10	NC	1	909.784	5
319		8	max	0	1	.006	2	0	3	2.8e-3	4	NC	1	NC	1
320			min	0	3	-.005	3	-.018	5	-1.647e-5	10	NC	1	1055.628	5
321		9	max	0	1	.005	2	0	3	2.8e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.016	5	-1.647e-5	10	NC	1	1245.645	5
323		10	max	0	1	.005	2	0	3	2.8e-3	4	NC	1	NC	1
324			min	0	3	-.004	3	-.013	5	-1.647e-5	10	NC	1	1499.897	5
325		11	max	0	1	.004	2	0	3	2.8e-3	4	NC	1	NC	1
326			min	0	3	-.004	3	-.01	5	-1.647e-5	10	NC	1	1851.42	5
327		12	max	0	1	.004	2	0	3	2.8e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.003	3	-.008	5	-1.647e-5	10	NC	1	2357.834	5
329		13	max	0	1	.003	2	0	3	2.8e-3	4	NC	1	NC	1
330			min	0	3	-.003	3	-.006	5	-1.647e-5	10	NC	1	3127.208	5
331		14	max	0	1	.003	2	0	3	2.8e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.004	5	-1.647e-5	10	NC	1	4382.436	5
333		15	max	0	1	.002	2	0	3	2.8e-3	4	NC	1	NC	1
334			min	0	3	-.002	3	-.003	5	-1.647e-5	10	NC	1	6647.126	5
335		16	max	0	1	.002	2	0	3	2.8e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.002	5	-1.647e-5	10	NC	1	NC	1
337		17	max	0	1	.001	2	0	3	2.8e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-1.647e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	3	2.8e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-1.647e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.8e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.647e-5	10	NC	1	NC	1
343	M1	1	max	.007	3	.024	3	.005	5	4.933e-3	2	NC	1	NC	1
344			min	-.008	2	-.019	2	0	9	-7.176e-3	3	NC	1	NC	1
345		2	max	.007	3	.014	3	.007	5	2.441e-3	2	NC	4	NC	1
346			min	-.008	2	-.011	2	-.001	9	-3.521e-3	3	4775.127	3	NC	1
347		3	max	.007	3	.004	3	.008	5	2.186e-4	5	NC	4	NC	1
348			min	-.008	2	-.003	2	-.002	9	-7.249e-5	9	2476.418	3	NC	1
349		4	max	.007	3	.004	2	.01	5	2.15e-4	5	NC	4	NC	1
350			min	-.008	2	-.004	3	-.002	9	-6.044e-5	9	1768.714	3	8716.069	5
351		5	max	.007	3	.01	2	.013	5	2.114e-4	5	NC	4	NC	1
352			min	-.008	2	-.01	3	-.002	1	-4.838e-5	9	1432.064	3	6171.568	5
353		6	max	.007	3	.016	2	.015	5	2.077e-4	5	NC	4	NC	1
354			min	-.008	2	-.015	3	-.002	9	-3.632e-5	9	1244.934	3	4702.785	5
355		7	max	.007	3	.02	2	.017	5	2.041e-4	5	NC	4	NC	1
356			min	-.008	2	-.019	3	-.001	9	-2.427e-5	9	1134.991	3	3761.363	5
357		8	max	.007	3	.023	2	.02	5	2.005e-4	5	NC	4	NC	1
358			min	-.008	2	-.022	3	-.001	9	-1.221e-5	9	1053.545	2	3114.975	5
359		9	max	.007	3	.025	2	.023	5	1.986e-4	4	NC	4	NC	1
360			min	-.008	2	-.023	3	0	9	-1.428e-6	10	1001.782	2	2649.028	5
361		10	max	.007	3	.026	2	.025	4	1.989e-4	4	NC	4	NC	1
362			min	-.008	2	-.024	3	0	9	-3.255e-6	10	977.807	2	2287.313	4
363		11	max	.007	3	.025	2	.028	4	1.992e-4	4	NC	4	NC	1
364			min	-.008	2	-.023	3	0	10	-5.081e-6	10	978.805	2	2010.308	4
365		12	max	.007	3	.024	2	.031	4	1.996e-4	4	NC	4	NC	1
366			min	-.008	2	-.021	3	0	10	-6.908e-6	10	1005.903	2	1795.553	4
367		13	max	.007	3	.021	2	.034	4	1.999e-4	4	NC	4	NC	1
368			min	-.008	2	-.018	3	0	10	-8.734e-6	10	1064.779	2	1626.297	4
369		14	max	.007	3	.016	2	.036	4	2.002e-4	4	NC	4	NC	1
370			min	-.008	2	-.014	3	0	10	-1.056e-5	10	1168.55	2	1491.302	4
371		15	max	.007	3	.011	2	.039	4	2.005e-4	4	NC	4	NC	1
372			min	-.008	2	-.009	3	0	10	-1.239e-5	10	1346.005	2	1382.814	4
373		16	max	.007	3	.004	2	.041	4	3.455e-4	4	NC	4	NC	1
374			min	-.008	2	-.003	3	0	10	-1.376e-5	10	1667.551	2	1295.351	4
375		17	max	.007	3	.003	3	.043	4	3.934e-3	4	NC	4	NC	1
376			min	-.008	2	-.005	2	0	10	-4.279e-6	10	2358.326	2	1225.067	4
377		18	max	.007	3	.011	3	.045	4	3.415e-3	2	NC	4	NC	1
378			min	-.008	2	-.015	2	0	10	-1.815e-3	3	4567.617	2	1168.889	4
379		19	max	.007	3	.019	3	.047	4	6.89e-3	2	NC	1	NC	1
380			min	-.008	2	-.025	2	0	9	-3.735e-3	3	NC	1	1125.989	4
381	M5	1	max	.021	3	.072	3	.005	5	1.678e-5	4	NC	1	NC	1
382			min	-.024	2	-.06	2	0	9	7.836e-8	11	NC	1	NC	1
383		2	max	.021	3	.041	3	.006	5	1.273e-4	3	NC	4	NC	1
384			min	-.024	2	-.034	2	0	9	-7.772e-6	9	1558.037	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

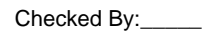
Dec 11, 2015

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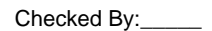
### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.021	3	.012	3	.008	5	2.427e-4	3	NC	5	NC	1
386		min	-.024	2	-.009	2	0	9	-1.546e-5	9	808.555	3	NC	1
387	4	max	.021	3	.013	2	.01	5	2.36e-4	3	NC	5	NC	1
388		min	-.024	2	-.012	3	0	9	-1.457e-5	9	578.407	3	NC	1
389	5	max	.021	3	.032	2	.013	5	2.292e-4	3	NC	5	NC	1
390		min	-.024	2	-.031	3	0	9	-1.369e-5	9	469.097	3	9605.395	3
391	6	max	.021	3	.048	2	.015	5	2.225e-4	3	NC	5	NC	1
392		min	-.024	2	-.047	3	0	9	-1.28e-5	9	408.494	3	8680.684	3
393	7	max	.021	3	.061	2	.018	5	2.219e-4	5	NC	5	NC	1
394		min	-.024	2	-.059	3	0	9	-1.191e-5	9	368.468	2	8262.602	3
395	8	max	.021	3	.071	2	.021	5	2.274e-4	5	NC	5	NC	1
396		min	-.024	2	-.067	3	0	9	-1.102e-5	9	340.213	2	8183.291	3
397	9	max	.021	3	.077	2	.024	4	2.329e-4	5	NC	5	NC	1
398		min	-.024	2	-.071	3	0	9	-1.013e-5	9	323.474	2	8380.657	3
399	10	max	.021	3	.079	2	.027	4	2.384e-4	5	NC	5	NC	1
400		min	-.024	2	-.072	3	0	9	-9.245e-6	9	315.723	2	8849.641	3
401	11	max	.021	3	.078	2	.03	4	2.439e-4	5	NC	5	NC	1
402		min	-.024	2	-.07	3	0	9	-8.357e-6	9	316.05	2	9630.661	3
403	12	max	.021	3	.073	2	.033	4	2.495e-4	4	NC	5	NC	1
404		min	-.024	2	-.064	3	0	9	-7.469e-6	9	324.816	2	NC	1
405	13	max	.021	3	.064	2	.035	4	2.554e-4	4	NC	5	NC	1
406		min	-.024	2	-.055	3	0	9	-6.58e-6	9	343.858	2	NC	1
407	14	max	.021	3	.051	2	.038	4	2.614e-4	4	NC	5	NC	1
408		min	-.024	2	-.043	3	0	9	-5.692e-6	9	377.413	2	NC	1
409	15	max	.02	3	.033	2	.04	4	2.673e-4	4	NC	5	NC	1
410		min	-.024	2	-.028	3	0	9	-4.804e-6	9	434.785	2	NC	1
411	16	max	.02	3	.012	2	.042	4	4.154e-4	4	NC	5	NC	1
412		min	-.024	2	-.01	3	0	9	-4.52e-6	9	538.716	2	NC	1
413	17	max	.02	3	.01	3	.044	4	3.949e-3	4	NC	5	NC	1
414		min	-.024	2	-.015	2	0	9	-1.863e-5	9	761.901	2	NC	1
415	18	max	.02	3	.032	3	.046	4	2.029e-3	4	NC	4	NC	1
416		min	-.024	2	-.046	2	0	9	-9.569e-6	9	1475.866	2	NC	1
417	19	max	.02	3	.055	3	.047	4	6.226e-6	5	NC	1	NC	1
418		min	-.024	2	-.078	2	0	9	-1.497e-6	3	NC	1	NC	1
419	M9	1	max	.007	.023	.005	.005	5	7.195e-3	3	NC	1	NC	1
420		min	-.008	2	-.019	2	0	9	-4.932e-3	2	NC	1	NC	1
421	2	max	.007	3	.013	.004	.004	4	3.554e-3	3	NC	4	NC	1
422		min	-.008	2	-.011	2	0	10	-2.44e-3	2	4777.882	3	NC	1
423	3	max	.007	3	.004	.004	.004	4	7.258e-5	1	NC	4	NC	1
424		min	-.008	2	-.003	2	0	10	-3.031e-5	5	2477.874	3	NC	1
425	4	max	.007	3	.004	.005	.005	4	5.789e-5	1	NC	4	NC	1
426		min	-.008	2	-.004	3	-.001	3	-3.229e-5	5	1769.736	3	NC	1
427	5	max	.007	3	.01	.006	.006	4	4.32e-5	1	NC	4	NC	1
428		min	-.008	2	-.011	3	-.002	3	-3.759e-5	3	1432.841	3	9243.29	3
429	6	max	.007	3	.016	.008	.008	4	2.852e-5	1	NC	4	NC	1
430		min	-.008	2	-.016	3	-.003	3	-4.605e-5	3	1245.553	3	8057.902	3
431	7	max	.007	3	.02	.01	.01	4	1.383e-5	1	NC	4	NC	1
432		min	-.008	2	-.02	3	-.004	3	-5.45e-5	3	1135.497	3	7379.438	3
433	8	max	.007	3	.023	.012	.012	4	-2.517e-7	10	NC	4	NC	1
434		min	-.008	2	-.022	3	-.005	3	-6.296e-5	3	1053.788	2	6257.127	4
435	9	max	.007	3	.025	.015	.015	4	1.566e-6	10	NC	4	NC	1
436		min	-.008	2	-.024	3	-.005	3	-7.142e-5	3	1002.021	2	4624.034	4
437	10	max	.007	3	.026	.017	.017	4	3.384e-6	10	NC	4	NC	1
438		min	-.008	2	-.024	3	-.005	3	-7.988e-5	3	978.048	2	3597.399	4
439	11	max	.007	3	.025	.021	.021	5	5.202e-6	10	NC	4	NC	1
440		min	-.008	2	-.023	3	-.005	3	-8.834e-5	3	979.053	2	2907.893	4
441	12	max	.007	3	.024	.024	.024	5	7.02e-6	10	NC	4	NC	1





RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 25° 130mph 30psf 3.5ft 7-05.rdb Page 40



RISA-3D Version 13.0.0    \...\...\PVMMini 60 Cell 1V 25° 130mph 30psf 3.5ft 7-05.r    Page 41



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.004	4	-.006	9	-.036	3	-4.154e-3	2	7705.791	1	1086.164	3
557	13	max	.001	3	.005	5	.026	2	5.013e-3	3	NC	3	7240.164	15
558		min	-.005	4	-.006	9	-.035	3	-4.527e-3	2	8344.968	1	1075.232	3
559	14	max	.001	3	.005	5	.023	2	5.399e-3	3	NC	3	9703.707	15
560		min	-.005	4	-.005	9	-.032	3	-4.901e-3	2	9409.995	1	1108.606	3
561	15	max	.001	3	.005	5	.019	1	5.784e-3	3	NC	1	NC	7
562		min	-.005	4	-.005	9	-.027	3	-5.274e-3	2	NC	1	1203.492	3
563	16	max	.001	3	.005	5	.014	1	6.169e-3	3	NC	1	NC	5
564		min	-.006	4	-.004	9	-.018	3	-5.648e-3	2	NC	1	1406.638	3
565	17	max	.002	3	.005	5	.006	1	6.554e-3	3	NC	1	NC	4
566		min	-.006	4	-.003	9	-.007	3	-6.021e-3	2	NC	1	1864.732	3
567	18	max	.002	3	.005	5	.007	3	6.94e-3	3	NC	1	NC	4
568		min	-.006	4	-.002	9	-.01	2	-6.395e-3	2	NC	1	3319.843	3
569	19	max	.002	3	.005	2	.025	3	7.325e-3	3	NC	1	NC	1
570		min	-.007	4	0	9	-.025	2	-6.768e-3	2	NC	1	NC	1
571	M16A	1	max	.001	.002	2	.008	3	2.109e-3	3	NC	1	NC	1
572		min	-.002	4	-.003	4	-.008	2	-2.177e-3	2	NC	1	NC	1
573	2	max	.001	2	0	2	.002	3	2.028e-3	3	NC	1	NC	1
574		min	-.002	4	-.006	4	-.003	2	-2.077e-3	2	NC	1	9144.774	3
575	3	max	.001	2	-.001	10	.003	1	1.948e-3	3	NC	1	NC	4
576		min	-.002	4	-.008	4	-.004	5	-1.976e-3	2	NC	1	5174.686	3
577	4	max	0	2	-.002	10	.005	1	1.867e-3	3	NC	1	NC	4
578		min	-.002	4	-.01	4	-.007	3	-1.876e-3	2	7319.697	4	3936.234	3
579	5	max	0	2	-.003	12	.007	1	1.787e-3	3	NC	1	NC	9
580		min	-.002	4	-.012	4	-.01	3	-1.776e-3	2	5711.633	4	3400.005	3
581	6	max	0	2	-.003	12	.008	1	1.706e-3	3	NC	3	NC	9
582		min	-.002	4	-.014	4	-.013	5	-1.675e-3	2	4806.943	4	3166.449	3
583	7	max	0	2	-.004	12	.008	1	1.626e-3	3	NC	3	NC	9
584		min	-.002	4	-.015	4	-.016	5	-1.575e-3	2	4262.891	4	3110.493	3
585	8	max	0	2	-.004	12	.008	1	1.545e-3	3	NC	3	NC	9
586		min	-.001	4	-.016	4	-.019	5	-1.474e-3	2	3936.379	4	3155.261	5
587	9	max	0	2	-.004	12	.008	1	1.465e-3	3	NC	12	NC	9
588		min	-.001	4	-.017	4	-.02	5	-1.374e-3	2	3760.63	4	2838.517	5
589	10	max	0	2	-.004	12	.007	1	1.384e-3	3	NC	12	NC	9
590		min	-.001	4	-.017	4	-.022	5	-1.274e-3	2	3705.032	4	2678.642	5
591	11	max	0	2	-.004	12	.006	1	1.304e-3	3	NC	12	NC	9
592		min	-.001	4	-.016	4	-.022	5	-1.173e-3	2	3760.63	4	2641.779	5
593	12	max	0	2	-.004	12	.005	1	1.223e-3	3	NC	3	NC	9
594		min	0	4	-.015	4	-.021	5	-1.073e-3	2	3936.379	4	2721.564	5
595	13	max	0	2	-.004	12	.004	1	1.143e-3	3	NC	3	NC	1
596		min	0	4	-.014	4	-.019	5	-9.725e-4	2	4262.891	4	2936.569	5
597	14	max	0	2	-.003	12	.003	1	1.062e-3	3	NC	3	NC	1
598		min	0	4	-.012	4	-.017	5	-8.721e-4	2	4806.943	4	3340.943	5
599	15	max	0	2	-.003	12	.001	1	9.816e-4	3	NC	1	NC	1
600		min	0	4	-.01	4	-.014	5	-7.718e-4	2	5711.633	4	4060.64	5
601	16	max	0	2	-.002	12	0	9	9.011e-4	3	NC	1	NC	1
602		min	0	4	-.008	4	-.011	5	-6.714e-4	2	7319.697	4	5409.673	5
603	17	max	0	2	-.001	12	0	9	8.206e-4	3	NC	1	NC	1
604		min	0	4	-.006	4	-.007	5	-5.71e-4	2	NC	1	8365.407	5
605	18	max	0	2	0	12	0	3	8.181e-4	4	NC	1	NC	1
606		min	0	4	-.003	4	-.003	5	-4.706e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	8.758e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-3.703e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

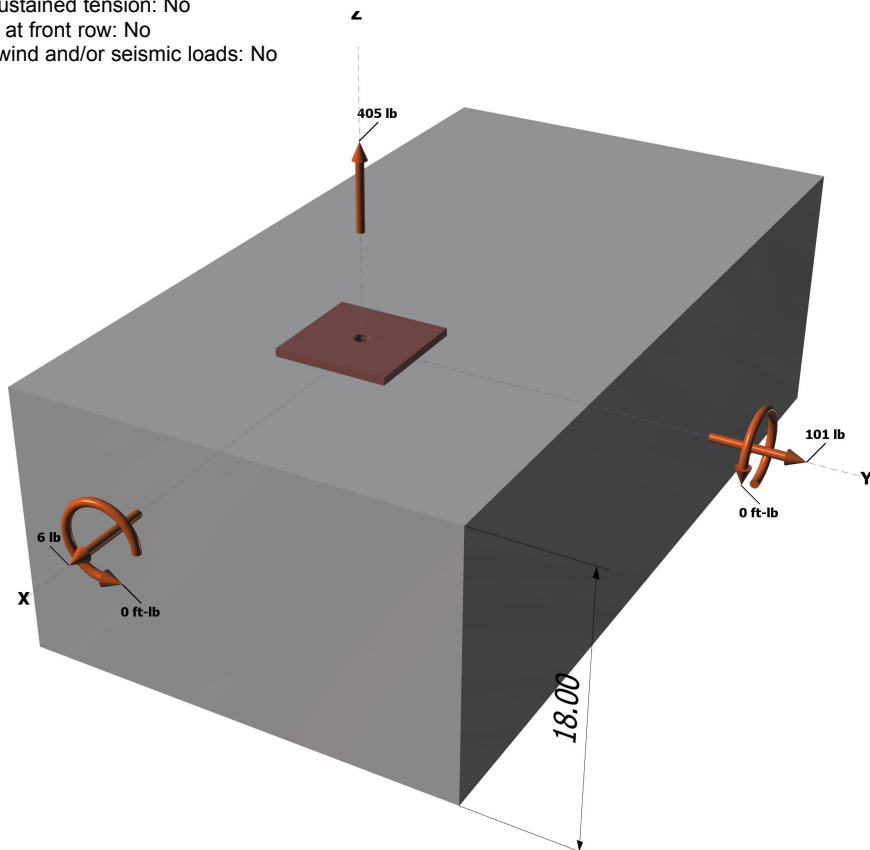
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



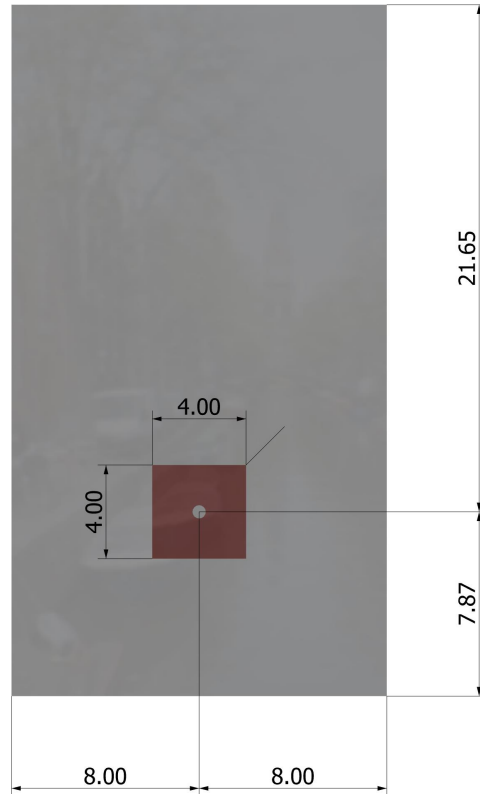
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

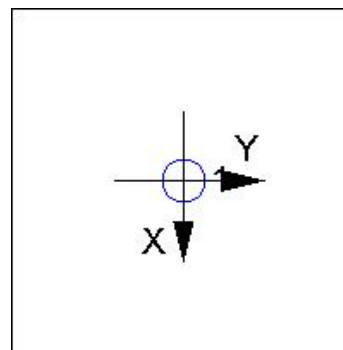
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMINI - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

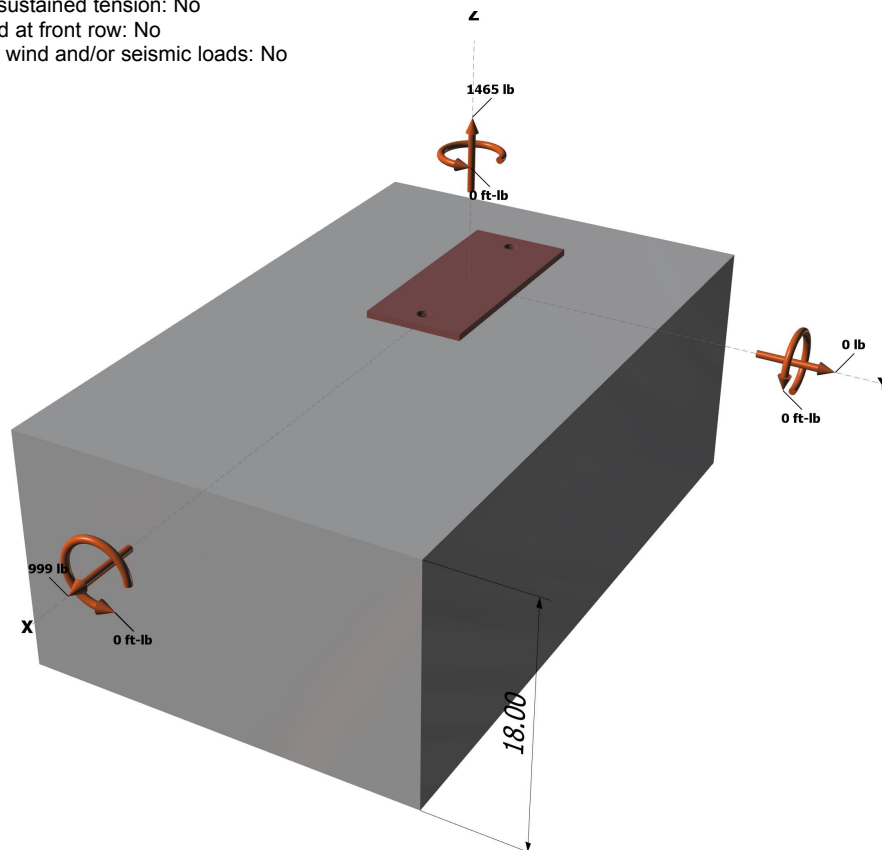
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



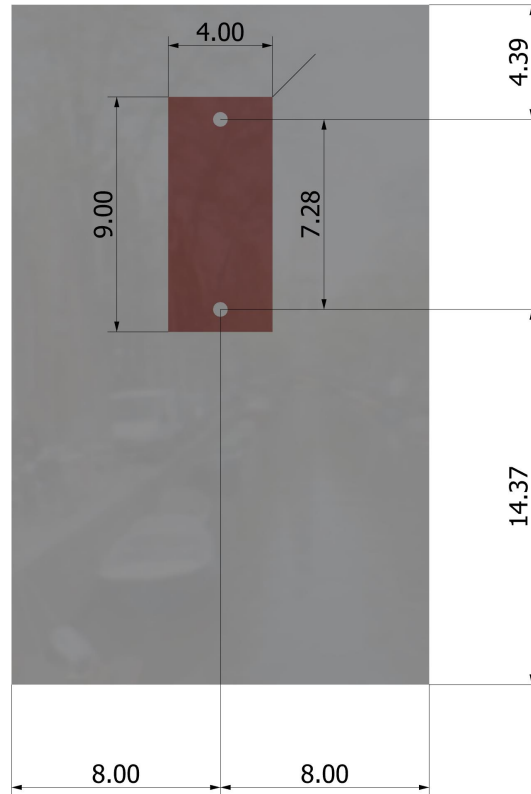
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

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- Refer to manufacturer's product literature for hole cleaning and installation instructions.