

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

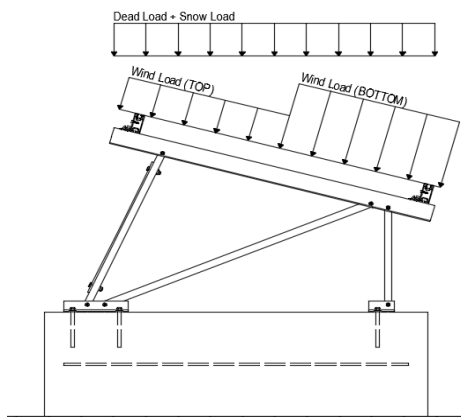
### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

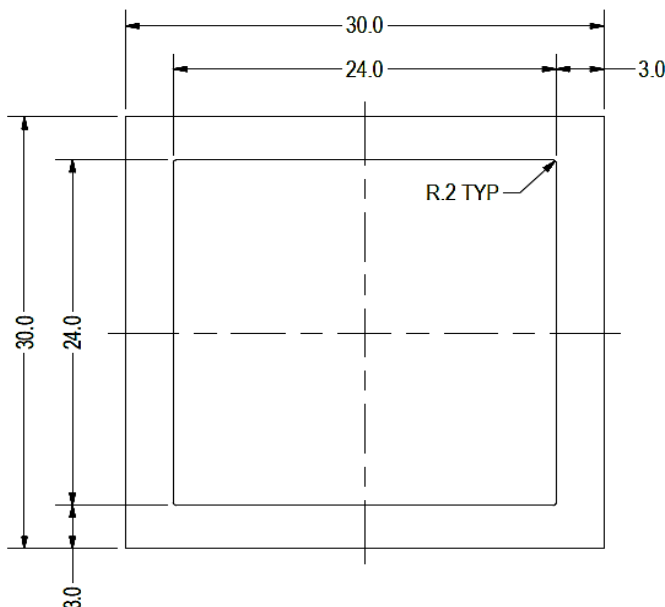
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

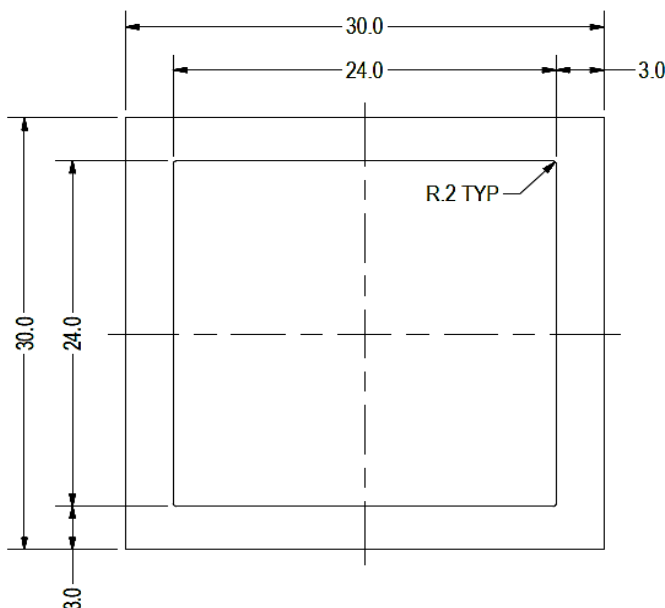
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.846 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

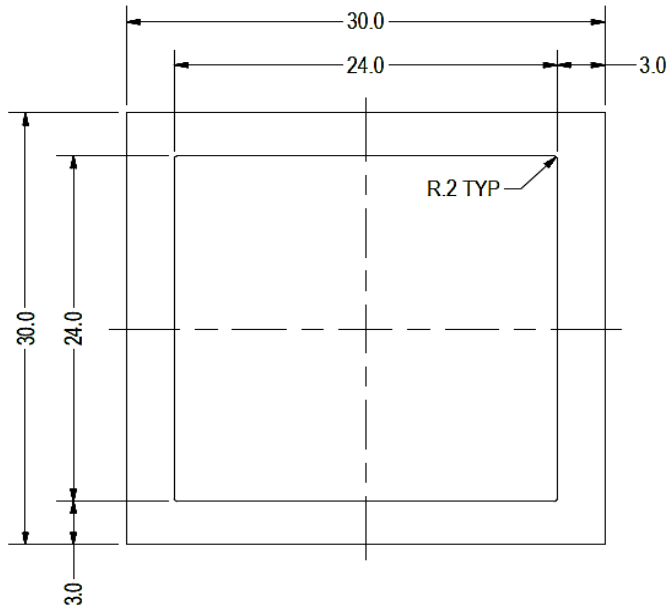
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.624 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

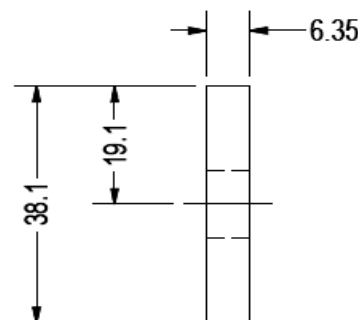
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.700 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>14%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.092 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>5%</b>



A cross brace kit is required every 38 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

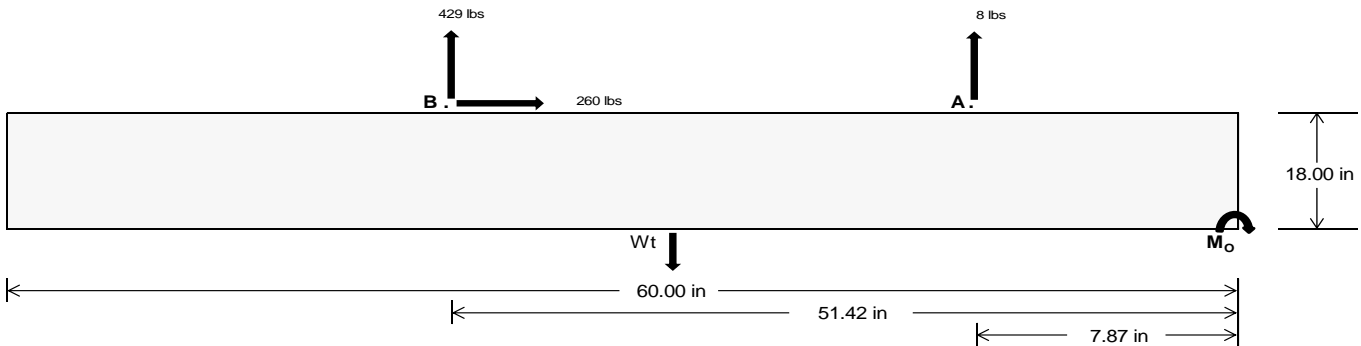
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>38.35</u>	<u>1784.76</u>	k
Compressive Load =	<u>1099.40</u>	<u>1210.43</u>	k
Lateral Load =	<u>1.86</u>	<u>1080.09</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 26772.3$  in-lbs  
Resisting Force Required = 892.41 lbs  
S.F. = 1.67  
Weight Required = 1487.35 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 259.57 lbs  
Friction = 0.4  
Weight Required = 648.93 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 259.57 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	380 lbs	380 lbs	380 lbs	380 lbs	402 lbs	402 lbs	402 lbs	402 lbs	552 lbs	552 lbs	552 lbs	552 lbs	-17 lbs	-17 lbs	-17 lbs	-17 lbs
$F_B$	260 lbs	260 lbs	260 lbs	260 lbs	528 lbs	528 lbs	528 lbs	528 lbs	567 lbs	567 lbs	567 lbs	567 lbs	-857 lbs	-857 lbs	-857 lbs	-857 lbs
$F_V$	39 lbs	39 lbs	39 lbs	39 lbs	467 lbs	467 lbs	467 lbs	467 lbs	376 lbs	376 lbs	376 lbs	376 lbs	-519 lbs	-519 lbs	-519 lbs	-519 lbs
$P_{total}$	2634 lbs	2725 lbs	2815 lbs	2906 lbs	2924 lbs	3014 lbs	3105 lbs	3195 lbs	3112 lbs	3203 lbs	3293 lbs	3384 lbs	323 lbs	377 lbs	431 lbs	486 lbs
$M$	294 lbs-ft	294 lbs-ft	294 lbs-ft	294 lbs-ft	498 lbs-ft	498 lbs-ft	498 lbs-ft	498 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft
$e$	0.11 ft	0.11 ft	0.10 ft	0.10 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.23 ft	1.91 ft	1.67 ft	1.48 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	248.8 psf	247.4 psf	246.2 psf	245.0 psf	253.7 psf	252.1 psf	250.7 psf	249.3 psf	264.9 psf	262.8 psf	261.0 psf	259.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	325.9 psf	321.2 psf	316.9 psf	312.9 psf	384.2 psf	376.9 psf	370.3 psf	364.2 psf	414.1 psf	405.5 psf	397.7 psf	390.5 psf	439.9 psf	222.6 psf	173.3 psf	152.8 psf

Maximum Bearing Pressure = 440 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

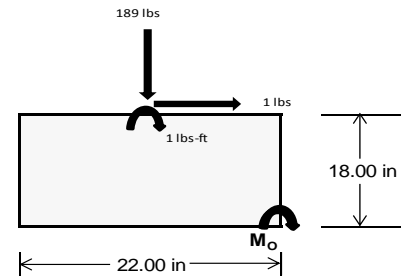
## Overturning Check

$M_o = 171.2 \text{ ft-lbs}$   
 Resisting Force Required = 186.75 lbs  
 S.F. = 1.67  
 Weight Required = 311.26 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	60 lbs	147 lbs	56 lbs	189 lbs	531 lbs	186 lbs	17 lbs	43 lbs	17 lbs
$F_v$	0 lbs	0 lbs	0 lbs	1 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2528 lbs	2616 lbs	2525 lbs	2539 lbs	2881 lbs	2536 lbs	739 lbs	765 lbs	738 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	275.7 sqft	285.3 sqft	275.4 sqft	276.1 sqft	314.0 sqft	276.4 sqft	80.6 sqft	83.4 sqft	80.5 sqft
$f_{max}$	275.9 psf	285.4 psf	275.5 psf	277.8 psf	314.6 psf	276.8 psf	80.7 psf	83.5 psf	80.6 psf



Maximum Bearing Pressure = 315 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



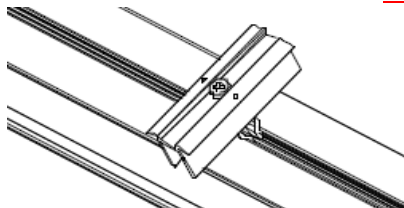
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

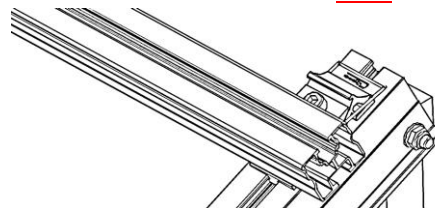
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.657 k
Allowable Uplift =	1.214 k
Utilization =	<u>54%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.112 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



### 6.2 Bolted Connections

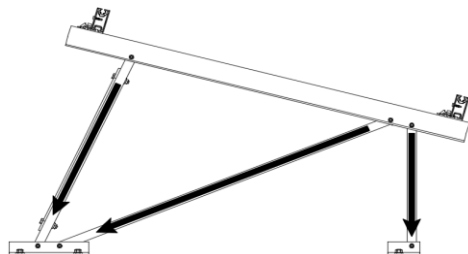
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.846 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>15%</u>

#### Diagonal Strut

Maximum Axial Load =	0.624 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.146 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.092 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.011 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$148.425$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$154.13$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.251 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L S_t = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} S_t = 0.408 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L W_k = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} W_k = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	224.341	2	286.485	2	0	15	0	15	0	1	0	1
2		min	-268.861	3	-426.527	3	-.136	3	0	3	0	1	0	1
3	N7	max	.002	3	310.925	1	-.029	15	0	15	0	1	0	1
4		min	-.142	2	2.99	12	-.689	1	-.001	1	0	1	0	1
5	N15	max	0	15	845.692	1	.278	1	0	1	0	1	0	1
6		min	-1.431	2	-29.501	3	-.616	3	0	3	0	1	0	1
7	N16	max	760.991	2	931.099	2	0	2	0	9	0	1	0	1
8		min	-830.839	3	-1372.891	3	-74.433	3	0	3	0	1	0	1
9	N23	max	.002	3	310.904	1	1.316	1	.002	1	0	1	0	1
10		min	-.142	2	3.38	12	.039	10	0	10	0	1	0	1
11	N24	max	224.341	2	289.178	2	75.04	3	0	1	0	1	0	1
12		min	-269.269	3	-425.268	3	.001	10	0	3	0	1	0	1
13	Totals:	max	1207.959	2	2784.822	2	0	1						
14		min	-1369.011	3	-2246.471	3	0	3						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	220.364	1	.655	4	.201	1	0	15	0	15	0	1
2			min	-366.808	3	.154	15	-.065	3	0	1	0	1	0	1
3		2	max	220.49	1	.604	4	.201	1	0	15	0	15	0	15
4			min	-366.714	3	.142	15	-.065	3	0	1	0	1	0	4
5		3	max	220.616	1	.553	4	.201	1	0	15	0	15	0	15
6			min	-366.619	3	.13	15	-.065	3	0	1	0	3	0	4
7		4	max	220.742	1	.502	4	.201	1	0	15	0	9	0	15
8			min	-366.525	3	.118	15	-.065	3	0	1	0	3	0	4
9		5	max	220.868	1	.451	4	.201	1	0	15	0	1	0	15
10			min	-366.431	3	.106	15	-.065	3	0	1	0	3	0	4
11		6	max	220.993	1	.4	4	.201	1	0	15	0	1	0	15
12			min	-366.336	3	.094	15	-.065	3	0	1	0	3	0	4
13		7	max	221.119	1	.348	4	.201	1	0	15	0	1	0	15
14			min	-366.242	3	.082	15	-.065	3	0	1	0	3	0	4
15		8	max	221.245	1	.297	4	.201	1	0	15	0	1	0	15
16			min	-366.147	3	.07	15	-.065	3	0	1	0	3	0	4
17		9	max	221.371	1	.246	4	.201	1	0	15	0	1	0	15
18			min	-366.053	3	.058	15	-.065	3	0	1	0	3	0	4
19		10	max	221.497	1	.195	4	.201	1	0	15	0	1	0	15
20			min	-365.959	3	.046	15	-.065	3	0	1	0	3	0	4
21		11	max	221.623	1	.144	4	.201	1	0	15	0	1	0	15
22			min	-365.864	3	.032	12	-.065	3	0	1	0	3	0	4
23		12	max	221.749	1	.103	2	.201	1	0	15	0	1	0	15
24			min	-365.77	3	.012	12	-.065	3	0	1	0	3	0	4
25		13	max	221.874	1	.063	2	.201	1	0	15	0	1	0	15
26			min	-365.675	3	-.014	3	-.065	3	0	1	0	3	0	4
27		14	max	222	1	.023	2	.201	1	0	15	0	1	0	15
28			min	-365.581	3	-.044	3	-.065	3	0	1	0	3	0	4
29		15	max	222.126	1	-.014	15	.201	1	0	15	0	1	0	15
30			min	-365.487	3	-.074	3	-.065	3	0	1	0	3	0	4
31		16	max	222.252	1	-.026	15	.201	1	0	15	0	1	0	15
32			min	-365.392	3	-.112	4	-.065	3	0	1	0	3	0	4
33		17	max	222.378	1	-.038	15	.201	1	0	15	0	1	0	15
34			min	-365.298	3	-.163	4	-.065	3	0	1	0	3	0	4
35		18	max	222.504	1	-.05	15	.201	1	0	15	0	1	0	15
36			min	-365.203	3	-.214	4	-.065	3	0	1	0	3	0	4
37		19	max	222.63	1	-.062	15	.201	1	0	15	0	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-365.109	3	-.265	4	-.065	3	0	1	0	3	0	4
39	M3	1	max	179.651	2	1.758	4	-.009	15	0	15	0	1	4
40		min	-176.786	3	.414	15	-.225	1	0	1	0	15	0	15
41		2	max	179.582	2	1.582	4	-.009	15	0	15	0	1	2
42		min	-176.838	3	.372	15	-.225	1	0	1	0	15	0	12
43		3	max	179.512	2	1.405	4	-.009	15	0	15	0	1	2
44		min	-176.89	3	.33	15	-.225	1	0	1	0	15	0	3
45		4	max	179.443	2	1.228	4	-.009	15	0	15	0	1	15
46		min	-176.942	3	.289	15	-.225	1	0	1	0	15	0	4
47		5	max	179.374	2	1.051	4	-.009	15	0	15	0	1	15
48		min	-176.994	3	.247	15	-.225	1	0	1	0	15	0	4
49		6	max	179.304	2	.874	4	-.009	15	0	15	0	1	15
50		min	-177.046	3	.206	15	-.225	1	0	1	0	15	0	4
51		7	max	179.235	2	.697	4	-.009	15	0	15	0	1	15
52		min	-177.098	3	.164	15	-.225	1	0	1	0	15	0	4
53		8	max	179.166	2	.521	4	-.009	15	0	15	0	1	15
54		min	-177.15	3	.123	15	-.225	1	0	1	0	15	-.001	4
55		9	max	179.096	2	.344	4	-.009	15	0	15	0	1	15
56		min	-177.202	3	.081	15	-.225	1	0	1	0	15	-.001	4
57		10	max	179.027	2	.167	4	-.009	15	0	15	0	1	15
58		min	-177.254	3	.039	15	-.225	1	0	1	0	15	-.001	4
59		11	max	178.958	2	.018	2	-.009	15	0	15	0	1	15
60		min	-177.306	3	-.038	3	-.225	1	0	1	0	15	-.001	4
61		12	max	178.888	2	-.044	15	-.009	15	0	15	0	1	15
62		min	-177.358	3	-.187	4	-.225	1	0	1	0	10	-.001	4
63		13	max	178.819	2	-.085	15	-.009	15	0	15	0	1	15
64		min	-177.41	3	-.364	4	-.225	1	0	1	0	10	-.001	4
65		14	max	178.75	2	-.127	15	-.009	15	0	15	0	1	15
66		min	-177.462	3	-.54	4	-.225	1	0	1	0	10	-.001	4
67		15	max	178.68	2	-.168	15	-.009	15	0	15	0	1	15
68		min	-177.514	3	-.717	4	-.225	1	0	1	0	10	0	4
69		16	max	178.611	2	-.21	15	-.009	15	0	15	0	9	15
70		min	-177.566	3	-.894	4	-.225	1	0	1	0	2	0	4
71		17	max	178.542	2	-.252	15	-.009	15	0	15	0	15	15
72		min	-177.618	3	-1.071	4	-.225	1	0	1	0	1	0	4
73		18	max	178.473	2	-.293	15	-.009	15	0	15	0	15	15
74		min	-177.67	3	-1.248	4	-.225	1	0	1	0	1	0	4
75		19	max	178.403	2	-.335	15	-.009	15	0	15	0	15	1
76		min	-177.722	3	-1.425	4	-.225	1	0	1	0	1	0	1
77	M4	1	max	309.76	1	0	1	-.029	15	0	1	0	3	1
78		min	2.408	12	0	1	-.727	1	0	1	0	2	0	1
79		2	max	309.825	1	0	1	-.029	15	0	1	0	15	1
80		min	2.44	12	0	1	-.727	1	0	1	0	1	0	1
81		3	max	309.89	1	0	1	-.029	15	0	1	0	15	1
82		min	2.473	12	0	1	-.727	1	0	1	0	1	0	1
83		4	max	309.954	1	0	1	-.029	15	0	1	0	15	1
84		min	2.505	12	0	1	-.727	1	0	1	0	1	0	1
85		5	max	310.019	1	0	1	-.029	15	0	1	0	15	1
86		min	2.537	12	0	1	-.727	1	0	1	0	1	0	1
87		6	max	310.084	1	0	1	-.029	15	0	1	0	15	1
88		min	2.57	12	0	1	-.727	1	0	1	0	1	0	1
89		7	max	310.148	1	0	1	-.029	15	0	1	0	15	1
90		min	2.602	12	0	1	-.727	1	0	1	0	1	0	1
91		8	max	310.213	1	0	1	-.029	15	0	1	0	15	1
92		min	2.635	12	0	1	-.727	1	0	1	0	1	0	1
93		9	max	310.278	1	0	1	-.029	15	0	1	0	15	1
94		min	2.667	12	0	1	-.727	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	310.343	1	0	1	-.029	15	0	1	0	15	0	1
96		min	2.699	12	0	1	-.727	1	0	1	0	1	0	1
97	11	max	310.407	1	0	1	-.029	15	0	1	0	15	0	1
98		min	2.732	12	0	1	-.727	1	0	1	0	1	0	1
99	12	max	310.472	1	0	1	-.029	15	0	1	0	15	0	1
100		min	2.764	12	0	1	-.727	1	0	1	0	1	0	1
101	13	max	310.537	1	0	1	-.029	15	0	1	0	15	0	1
102		min	2.796	12	0	1	-.727	1	0	1	0	1	0	1
103	14	max	310.601	1	0	1	-.029	15	0	1	0	15	0	1
104		min	2.829	12	0	1	-.727	1	0	1	0	1	0	1
105	15	max	310.666	1	0	1	-.029	15	0	1	0	15	0	1
106		min	2.861	12	0	1	-.727	1	0	1	0	1	0	1
107	16	max	310.731	1	0	1	-.029	15	0	1	0	15	0	1
108		min	2.893	12	0	1	-.727	1	0	1	0	1	0	1
109	17	max	310.796	1	0	1	-.029	15	0	1	0	15	0	1
110		min	2.926	12	0	1	-.727	1	0	1	-.001	1	0	1
111	18	max	310.86	1	0	1	-.029	15	0	1	0	15	0	1
112		min	2.958	12	0	1	-.727	1	0	1	-.001	1	0	1
113	19	max	310.925	1	0	1	-.029	15	0	1	0	15	0	1
114		min	2.99	12	0	1	-.727	1	0	1	-.001	1	0	1
115	M6	1	max	698.172	1	.656	.045	9	0	3	0	3	0	1
116		min	-1146.124	3	.154	15	-.234	3	0	2	0	2	0	1
117	2	max	698.298	1	.605	4	.045	9	0	3	0	3	0	15
118		min	-1146.029	3	.142	15	-.234	3	0	2	0	2	0	4
119	3	max	698.424	1	.554	4	.045	9	0	3	0	3	0	15
120		min	-1145.935	3	.13	15	-.234	3	0	2	0	2	0	4
121	4	max	698.55	1	.503	4	.045	9	0	3	0	3	0	15
122		min	-1145.84	3	.118	15	-.234	3	0	2	0	2	0	4
123	5	max	698.676	1	.452	4	.045	9	0	3	0	3	0	15
124		min	-1145.746	3	.102	12	-.234	3	0	2	0	2	0	4
125	6	max	698.802	1	.411	2	.045	9	0	3	0	1	0	15
126		min	-1145.652	3	.082	12	-.234	3	0	2	0	2	0	4
127	7	max	698.928	1	.371	2	.045	9	0	3	0	1	0	15
128		min	-1145.557	3	.062	12	-.234	3	0	2	0	3	0	4
129	8	max	699.054	1	.332	2	.045	9	0	3	0	1	0	15
130		min	-1145.463	3	.042	12	-.234	3	0	2	0	3	0	4
131	9	max	699.179	1	.292	2	.045	9	0	3	0	1	0	12
132		min	-1145.368	3	.023	12	-.234	3	0	2	0	3	0	4
133	10	max	699.305	1	.252	2	.045	9	0	3	0	1	0	12
134		min	-1145.274	3	-.003	3	-.234	3	0	2	0	3	0	2
135	11	max	699.431	1	.212	2	.045	9	0	3	0	1	0	12
136		min	-1145.18	3	-.033	3	-.234	3	0	2	0	3	0	2
137	12	max	699.557	1	.172	2	.045	9	0	3	0	1	0	12
138		min	-1145.085	3	-.063	3	-.234	3	0	2	0	3	0	2
139	13	max	699.683	1	.132	2	.045	9	0	3	0	1	0	12
140		min	-1144.991	3	-.093	3	-.234	3	0	2	0	3	0	2
141	14	max	699.809	1	.092	2	.045	9	0	3	0	1	0	12
142		min	-1144.896	3	-.123	3	-.234	3	0	2	0	3	0	2
143	15	max	699.935	1	.053	2	.045	9	0	3	0	1	0	12
144		min	-1144.802	3	-.153	3	-.234	3	0	2	0	3	0	2
145	16	max	700.06	1	.013	2	.045	9	0	3	0	1	0	12
146		min	-1144.708	3	-.183	3	-.234	3	0	2	0	3	0	2
147	17	max	700.186	1	-.027	2	.045	9	0	3	0	1	0	12
148		min	-1144.613	3	-.213	3	-.234	3	0	2	0	3	0	2
149	18	max	700.312	1	-.05	15	.045	9	0	3	0	1	0	3
150		min	-1144.519	3	-.243	3	-.234	3	0	2	0	3	0	2
151	19	max	700.438	1	-.062	15	.045	9	0	3	0	1	0	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1144.424	3	-.272	3	-.234	3	0	2	0	3	0	2
153		max	624.123	2	1.761	4	.031	3	0	1	0	1	0	2
154		min	-524.288	3	.414	15	-.009	1	0	3	0	3	0	3
155		2 max	624.054	2	1.584	4	.031	3	0	1	0	1	0	2
156		min	-524.34	3	.372	15	-.009	1	0	3	0	3	0	3
157		3 max	623.984	2	1.407	4	.031	3	0	1	0	1	0	2
158		min	-524.392	3	.331	15	-.009	1	0	3	0	3	0	3
159		4 max	623.915	2	1.23	4	.031	3	0	1	0	1	0	2
160		min	-524.444	3	.289	15	-.009	1	0	3	0	3	0	3
161		5 max	623.846	2	1.054	4	.031	3	0	1	0	1	0	15
162		min	-524.496	3	.248	15	-.009	1	0	3	0	3	0	3
163		6 max	623.776	2	.877	4	.031	3	0	1	0	1	0	15
164		min	-524.548	3	.206	15	-.009	1	0	3	0	3	0	4
165		7 max	623.707	2	.7	4	.031	3	0	1	0	1	0	15
166		min	-524.6	3	.165	15	-.009	1	0	3	0	3	0	4
167		8 max	623.638	2	.523	4	.031	3	0	1	0	1	0	15
168		min	-524.652	3	.123	15	-.009	1	0	3	0	3	-.001	4
169		9 max	623.568	2	.353	2	.031	3	0	1	0	1	0	15
170	M8	min	-524.704	3	.071	12	-.009	1	0	3	0	3	-.001	4
171		10 max	623.499	2	.216	2	.031	3	0	1	0	1	0	15
172		min	-524.756	3	-.003	3	-.009	1	0	3	0	3	-.001	4
173		11 max	623.43	2	.078	2	.031	3	0	1	0	1	0	15
174		min	-524.808	3	-.106	3	-.009	1	0	3	0	3	-.001	4
175		12 max	623.36	2	-.043	15	.031	3	0	1	0	1	0	15
176		min	-524.86	3	-.21	3	-.009	1	0	3	0	3	-.001	4
177		13 max	623.291	2	-.085	15	.031	3	0	1	0	1	0	15
178		min	-524.912	3	-.361	4	-.009	1	0	3	0	3	-.001	4
179		14 max	623.222	2	-.126	15	.031	3	0	1	0	1	0	15
180		min	-524.964	3	-.538	4	-.009	1	0	3	0	3	-.001	4
181		15 max	623.152	2	-.168	15	.031	3	0	1	0	1	0	15
182		min	-525.015	3	-.715	4	-.009	1	0	3	0	3	0	4
183		16 max	623.083	2	-.21	15	.031	3	0	1	0	1	0	15
184		min	-525.067	3	-.892	4	-.009	1	0	3	0	3	0	4
185		17 max	623.014	2	-.251	15	.031	3	0	1	0	1	0	15
186		min	-525.119	3	-1.068	4	-.009	1	0	3	0	3	0	4
187	M8	18 max	622.944	2	-.293	15	.031	3	0	1	0	1	0	15
188		min	-525.171	3	-1.245	4	-.009	1	0	3	0	3	0	4
189		19 max	622.875	2	-.334	15	.031	3	0	1	0	1	0	1
190		min	-525.223	3	-1.422	4	-.009	1	0	3	0	3	0	1
191		1 max	844.528	1	0	1	.323	1	0	1	0	2	0	1
192		min	-30.374	3	0	1	-.613	3	0	1	0	3	0	1
193		2 max	844.592	1	0	1	.323	1	0	1	0	1	0	1
194		min	-30.326	3	0	1	-.613	3	0	1	0	3	0	1
195		3 max	844.657	1	0	1	.323	1	0	1	0	1	0	1
196		min	-30.277	3	0	1	-.613	3	0	1	0	3	0	1
197		4 max	844.722	1	0	1	.323	1	0	1	0	1	0	1
198		min	-30.229	3	0	1	-.613	3	0	1	0	3	0	1
199		5 max	844.786	1	0	1	.323	1	0	1	0	1	0	1
200		min	-30.18	3	0	1	-.613	3	0	1	0	3	0	1
201		6 max	844.851	1	0	1	.323	1	0	1	0	1	0	1
202		min	-30.132	3	0	1	-.613	3	0	1	0	3	0	1
203		7 max	844.916	1	0	1	.323	1	0	1	0	1	0	1
204		min	-30.083	3	0	1	-.613	3	0	1	0	3	0	1
205	M8	8 max	844.981	1	0	1	.323	1	0	1	0	1	0	1
206		min	-30.034	3	0	1	-.613	3	0	1	0	3	0	1
207		9 max	845.045	1	0	1	.323	1	0	1	0	1	0	1
208		min	-29.986	3	0	1	-.613	3	0	1	0	3	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	845.11	1	0	1	.323	1	0	1	0	1	0	1
210			min	-29.937	3	0	1	-.613	3	0	1	0	3	0	1
211		11	max	845.175	1	0	1	.323	1	0	1	0	1	0	1
212			min	-29.889	3	0	1	-.613	3	0	1	0	3	0	1
213		12	max	845.239	1	0	1	.323	1	0	1	0	1	0	1
214			min	-29.84	3	0	1	-.613	3	0	1	0	3	0	1
215		13	max	845.304	1	0	1	.323	1	0	1	0	1	0	1
216			min	-29.792	3	0	1	-.613	3	0	1	0	3	0	1
217		14	max	845.369	1	0	1	.323	1	0	1	0	1	0	1
218			min	-29.743	3	0	1	-.613	3	0	1	0	3	0	1
219		15	max	845.433	1	0	1	.323	1	0	1	0	1	0	1
220			min	-29.695	3	0	1	-.613	3	0	1	0	3	0	1
221		16	max	845.498	1	0	1	.323	1	0	1	0	1	0	1
222			min	-29.646	3	0	1	-.613	3	0	1	0	3	0	1
223		17	max	845.563	1	0	1	.323	1	0	1	0	1	0	1
224			min	-29.598	3	0	1	-.613	3	0	1	0	3	0	1
225		18	max	845.628	1	0	1	.323	1	0	1	0	1	0	1
226			min	-29.549	3	0	1	-.613	3	0	1	0	3	0	1
227		19	max	845.692	1	0	1	.323	1	0	1	0	1	0	1
228			min	-29.501	3	0	1	-.613	3	0	1	0	3	0	1
229	M10	1	max	222.403	1	.655	4	-.004	15	0	1	0	1	0	1
230			min	-314.414	3	.154	15	-.12	1	0	3	0	3	0	1
231		2	max	222.529	1	.604	4	-.004	15	0	1	0	1	0	15
232			min	-314.319	3	.142	15	-.12	1	0	3	0	3	0	4
233		3	max	222.655	1	.553	4	-.004	15	0	1	0	1	0	15
234			min	-314.225	3	.13	15	-.12	1	0	3	0	3	0	4
235		4	max	222.781	1	.502	4	-.004	15	0	1	0	1	0	15
236			min	-314.13	3	.118	15	-.12	1	0	3	0	3	0	4
237		5	max	222.907	1	.451	4	-.004	15	0	1	0	1	0	15
238			min	-314.036	3	.106	15	-.12	1	0	3	0	3	0	4
239		6	max	223.033	1	.4	4	-.004	15	0	1	0	1	0	15
240			min	-313.942	3	.094	15	-.12	1	0	3	0	3	0	4
241		7	max	223.159	1	.348	4	-.004	15	0	1	0	1	0	15
242			min	-313.847	3	.082	15	-.12	1	0	3	0	3	0	4
243		8	max	223.284	1	.297	4	-.004	15	0	1	0	1	0	15
244			min	-313.753	3	.07	15	-.12	1	0	3	0	3	0	4
245		9	max	223.41	1	.246	4	-.004	15	0	1	0	9	0	15
246			min	-313.658	3	.058	15	-.12	1	0	3	0	3	0	4
247		10	max	223.536	1	.195	4	-.004	15	0	1	0	15	0	15
248			min	-313.564	3	.046	15	-.12	1	0	3	0	3	0	4
249		11	max	223.662	1	.144	4	-.004	15	0	1	0	15	0	15
250			min	-313.47	3	.034	15	-.12	1	0	3	0	3	0	4
251		12	max	223.788	1	.103	2	-.004	15	0	1	0	15	0	15
252			min	-313.375	3	.022	15	-.12	1	0	3	0	3	0	4
253		13	max	223.914	1	.063	2	-.004	15	0	1	0	15	0	15
254			min	-313.281	3	.006	12	-.12	1	0	3	0	3	0	4
255		14	max	224.04	1	.023	2	-.004	15	0	1	0	15	0	15
256			min	-313.186	3	-.022	3	-.12	1	0	3	0	3	0	4
257		15	max	224.165	1	-.014	15	-.004	15	0	1	0	15	0	15
258			min	-313.092	3	-.061	4	-.12	1	0	3	0	3	0	4
259		16	max	224.291	1	-.026	15	-.004	15	0	1	0	15	0	15
260			min	-312.998	3	-.112	4	-.12	1	0	3	0	3	0	4
261		17	max	224.417	1	-.038	15	-.004	15	0	1	0	15	0	15
262			min	-312.903	3	-.163	4	-.12	1	0	3	0	3	0	4
263		18	max	224.543	1	-.05	15	-.004	15	0	1	0	15	0	15
264			min	-312.809	3	-.214	4	-.12	1	0	3	0	3	0	4
265		19	max	224.669	1	-.062	15	-.004	15	0	1	0	15	0	15







Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380			min	-92.886	1	-162.846	3	-29.674	1	0	2	-.057	1	0	3
381	M5	1	max	217.883	1	1120.923	3	0	2	0	1	.009	3	0	3
382			min	3.179	12	-746.65	2	-67.266	3	0	3	0	10	0	2
383		2	max	218.023	1	1120.742	3	0	2	0	1	0	1	.161	2
384			min	3.249	12	-746.892	2	-67.266	3	0	3	-.006	3	-.243	3
385		3	max	262.454	3	5.182	9	7.447	3	0	3	0	1	.321	2
386			min	-51.801	2	-86.702	2	-.35	1	0	1	-.019	3	-.48	3
387		4	max	262.559	3	4.981	9	7.447	3	0	3	0	1	.34	2
388			min	-51.661	2	-86.944	2	-.35	1	0	1	-.018	3	-.47	3
389		5	max	262.664	3	4.779	9	7.447	3	0	3	0	1	.359	2
390			min	-51.522	2	-87.186	2	-.35	1	0	1	-.016	3	-.459	3
391		6	max	262.768	3	4.578	9	7.447	3	0	3	0	1	.377	2
392			min	-51.382	2	-87.428	2	-.35	1	0	1	-.015	3	-.448	3
393		7	max	262.873	3	4.376	9	7.447	3	0	3	0	1	.396	2
394			min	-51.243	2	-87.67	2	-.35	1	0	1	-.013	3	-.436	3
395		8	max	262.978	3	4.175	9	7.447	3	0	3	0	1	.416	2
396			min	-51.103	2	-87.911	2	-.35	1	0	1	-.011	3	-.425	3
397		9	max	263.083	3	3.973	9	7.447	3	0	3	0	1	.435	2
398			min	-50.963	2	-88.153	2	-.35	1	0	1	-.01	3	-.414	3
399		10	max	263.187	3	3.772	9	7.447	3	0	3	0	2	.454	2
400			min	-50.824	2	-88.395	2	-.35	1	0	1	-.008	3	-.403	3
401		11	max	263.292	3	3.57	9	7.447	3	0	3	0	2	.473	2
402			min	-50.684	2	-88.637	2	-.35	1	0	1	-.007	3	-.392	3
403		12	max	263.397	3	3.368	9	7.447	3	0	3	0	2	.492	2
404			min	-50.544	2	-88.879	2	-.35	1	0	1	-.005	3	-.381	3
405		13	max	263.501	3	3.167	9	7.447	3	0	3	0	2	.512	2
406			min	-50.405	2	-89.12	2	-.35	1	0	1	-.003	3	-.369	3
407		14	max	263.606	3	2.965	9	7.447	3	0	3	0	2	.531	2
408			min	-50.265	2	-89.362	2	-.35	1	0	1	-.002	3	-.358	3
409		15	max	263.711	3	2.764	9	7.447	3	0	3	0	2	.55	2
410			min	-50.126	2	-89.604	2	-.35	1	0	1	0	1	-.347	3
411		16	max	292.447	2	433.802	2	7.416	3	0	3	.001	3	.565	2
412			min	-23.968	3	-492.086	3	-.366	1	0	2	0	1	-.331	3
413		17	max	292.587	2	433.561	2	7.416	3	0	3	.003	3	.471	2
414			min	-23.863	3	-492.267	3	-.366	1	0	2	0	1	-.225	3
415		18	max	-5.909	12	1093.593	2	6.805	3	0	3	.004	3	.237	2
416			min	-218.041	1	-518.9	3	-.082	1	0	1	0	1	-.112	3
417		19	max	-5.84	12	1093.351	2	6.805	3	0	3	.006	3	0	3
418			min	-217.901	1	-519.082	3	-.082	1	0	1	0	1	0	2
419	M9	1	max	92.642	1	345.201	3	71.958	3	0	3	-.002	15	0	2
420			min	3.786	15	-231.579	2	1.254	15	0	2	-.056	1	0	3
421		2	max	92.781	1	345.02	3	71.958	3	0	3	0	12	.051	2
422			min	3.828	15	-231.821	2	1.254	15	0	2	-.05	1	-.075	3
423		3	max	89.169	3	4.896	9	28.15	1	0	1	.013	3	.1	2
424			min	-14.619	10	-24.646	2	-2.086	3	0	15	-.043	1	-.148	3
425		4	max	89.274	3	4.694	9	28.15	1	0	1	.013	3	.105	2
426			min	-14.503	10	-24.888	2	-2.086	3	0	15	-.037	1	-.146	3
427		5	max	89.378	3	4.493	9	28.15	1	0	1	.013	3	.111	2
428			min	-14.387	10	-25.13	2	-2.086	3	0	15	-.031	1	-.143	3
429		6	max	89.483	3	4.291	9	28.15	1	0	1	.012	3	.116	2
430			min	-14.27	10	-25.372	2	-2.086	3	0	15	-.024	1	-.14	3
431		7	max	89.588	3	4.09	9	28.15	1	0	1	.012	3	.122	2
432			min	-14.154	10	-25.614	2	-2.086	3	0	15	-.018	1	-.137	3
433		8	max	89.692	3	3.888	9	28.15	1	0	1	.011	3	.127	2
434			min	-14.038	10	-25.855	2	-2.086	3	0	15	-.012	1	-.134	3
435		9	max	89.797	3	3.687	9	28.15	1	0	1	.011	3	.133	2
436			min	-13.921	10	-26.097	2	-2.086	3	0	15	-.006	1	-.131	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	89.902	3	3.485	9	28.15	1	0	1	.01	3	.139	2
438		min	-13.805	10	-26.339	2	-2.086	3	0	15	0	1	-.127	3
439	11	max	90.007	3	3.284	9	28.15	1	0	1	.01	3	.144	2
440		min	-13.689	10	-26.581	2	-2.086	3	0	15	0	15	-.124	3
441	12	max	90.111	3	3.082	9	28.15	1	0	1	.012	1	.15	2
442		min	-13.572	10	-26.823	2	-2.086	3	0	15	0	15	-.121	3
443	13	max	90.216	3	2.881	9	28.15	1	0	1	.018	1	.156	2
444		min	-13.456	10	-27.064	2	-2.086	3	0	15	0	15	-.118	3
445	14	max	90.321	3	2.679	9	28.15	1	0	1	.024	1	.162	2
446		min	-13.339	10	-27.306	2	-2.086	3	0	15	.001	15	-.115	3
447	15	max	90.425	3	2.477	9	28.15	1	0	1	.03	1	.168	2
448		min	-13.223	10	-27.548	2	-2.086	3	0	15	.001	15	-.111	3
449	16	max	91.1	2	120.676	2	28.361	1	0	15	.037	1	.173	2
450		min	-6.717	3	-165.143	3	-2.126	3	0	3	.002	15	-.107	3
451	17	max	91.24	2	120.434	2	28.361	1	0	15	.043	1	.146	2
452		min	-6.612	3	-165.325	3	-2.126	3	0	3	.002	15	-.071	3
453	18	max	-3.828	15	338.912	2	29.761	1	0	2	.05	1	.074	2
454		min	-92.773	1	-162.656	3	-1.649	3	0	3	.002	15	-.036	3
455	19	max	-3.785	15	338.67	2	29.761	1	0	2	.056	1	0	2
456		min	-92.633	1	-162.837	3	-1.649	3	0	3	.002	15	0	3
457	M13	1	max	71.953	3	231.468	2	-3.786	15	0	.056	1	0	2
458		min	1.254	15	-345.235	3	-92.635	1	0	3	.002	15	0	3
459	2	max	71.953	3	164.326	2	-2.884	15	0	2	.013	1	.156	3
460		min	1.254	15	-244.609	3	-70.222	1	0	3	-.001	10	-.104	2
461	3	max	71.953	3	97.184	2	-1.982	15	0	2	.008	3	.258	3
462		min	1.254	15	-143.984	3	-47.81	1	0	3	-.018	1	-.173	2
463	4	max	71.953	3	30.041	2	-1.079	15	0	2	.005	3	.308	3
464		min	1.254	15	-43.358	3	-25.398	1	0	3	-.037	1	-.207	2
465	5	max	71.953	3	57.268	3	1.308	10	0	2	.003	3	.304	3
466		min	1.254	15	-37.101	2	-4.125	3	0	3	-.045	1	-.205	2
467	6	max	71.953	3	157.894	3	19.427	1	0	2	0	3	.247	3
468		min	1.254	15	-104.243	2	-2.812	3	0	3	-.041	1	-.168	2
469	7	max	71.953	3	258.52	3	41.839	1	0	2	0	12	.137	3
470		min	1.254	15	-171.386	2	-1.499	3	0	3	-.024	1	-.095	2
471	8	max	71.953	3	359.146	3	64.252	1	0	2	.005	2	.014	1
472		min	1.254	15	-238.528	2	-.186	3	0	3	0	3	-.026	3
473	9	max	71.953	3	459.772	3	86.664	1	0	2	.043	1	.157	2
474		min	1.254	15	-305.67	2	.912	12	0	3	0	3	-.242	3
475	10	max	71.953	3	560.398	3	109.076	1	0	2	.095	1	.336	2
476		min	1.254	15	-372.813	2	1.788	12	0	3	-.008	3	-.511	3
477	11	max	28.87	1	305.67	2	-.482	3	0	3	.043	1	.157	2
478		min	1.232	15	-459.772	3	-86.409	1	0	2	-.009	3	-.242	3
479	12	max	28.87	1	238.528	2	.831	3	0	3	.005	2	.014	1
480		min	1.232	15	-359.146	3	-63.997	1	0	2	-.009	3	-.026	3
481	13	max	28.87	1	171.386	2	2.144	3	0	3	-.001	15	.137	3
482		min	1.232	15	-258.52	3	-41.585	1	0	2	-.025	1	-.095	2
483	14	max	28.87	1	104.243	2	3.456	3	0	3	-.002	15	.247	3
484		min	1.232	15	-157.894	3	-19.173	1	0	2	-.041	1	-.168	2
485	15	max	28.87	1	37.101	2	4.769	3	0	3	-.002	15	.304	3
486		min	1.232	15	-57.268	3	-1.308	10	0	2	-.045	1	-.205	2
487	16	max	28.87	1	43.358	3	25.652	1	0	3	-.001	12	.308	3
488		min	1.232	15	-30.041	2	1.094	15	0	2	-.037	1	-.207	2
489	17	max	28.87	1	143.984	3	48.064	1	0	3	.002	3	.258	3
490		min	1.232	15	-97.184	2	1.996	15	0	2	-.018	1	-.173	2
491	18	max	28.87	1	244.61	3	70.477	1	0	3	.013	1	.156	3
492		min	1.232	15	-164.326	2	2.898	15	0	2	-.001	10	-.104	2
493	19	max	28.87	1	345.235	3	92.889	1	0	3	.057	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	1.232	15	-231.468	2	3.8	15	0	2	.002	15	0	3
495	M16	1	max	1.652	3	338.809	2	-3.785	15	0	3	.056	1	0	2
496			min	-29.696	1	-162.864	3	-92.641	1	0	2	.002	15	0	3
497		2	max	1.652	3	240.349	2	-2.883	15	0	3	.013	1	.074	3
498			min	-29.696	1	-115.925	3	-70.228	1	0	2	-.001	10	-.153	2
499		3	max	1.652	3	141.89	2	-1.981	15	0	3	0	12	.122	3
500			min	-29.696	1	-68.987	3	-47.816	1	0	2	-.018	1	-.254	2
501		4	max	1.652	3	43.43	2	-1.079	15	0	3	-.001	15	.146	3
502			min	-29.696	1	-22.048	3	-25.404	1	0	2	-.037	1	-.303	2
503		5	max	1.652	3	24.891	3	1.295	10	0	3	-.002	15	.146	3
504			min	-29.696	1	-55.029	2	-2.992	1	0	2	-.045	1	-.3	2
505		6	max	1.652	3	71.829	3	19.421	1	0	3	-.002	15	.12	3
506			min	-29.696	1	-153.488	2	-1.479	3	0	2	-.041	1	-.245	2
507		7	max	1.652	3	118.768	3	41.833	1	0	3	-.001	15	.07	3
508			min	-29.696	1	-251.948	2	-.166	3	0	2	-.024	1	-.138	2
509		8	max	1.652	3	165.707	3	64.245	1	0	3	.005	2	.021	2
510			min	-29.696	1	-350.407	2	.87	12	0	2	-.006	3	-.005	3
511		9	max	1.652	3	212.645	3	86.658	1	0	3	.043	1	.232	2
512			min	-29.696	1	-448.867	2	1.745	12	0	2	-.005	3	-.105	3
513		10	max	-1.266	15	-8.694	15	109.07	1	0	15	.095	1	.495	2
514			min	-29.696	1	-547.326	2	-4.734	3	0	2	-.003	3	-.23	3
515		11	max	-1.262	15	448.867	2	-2.344	12	0	2	.043	1	.232	2
516			min	-29.611	1	-212.645	3	-86.405	1	0	3	0	3	-.105	3
517		12	max	-1.262	15	350.407	2	-1.469	12	0	2	.005	2	.021	2
518			min	-29.611	1	-165.707	3	-63.993	1	0	3	-.001	3	-.005	3
519		13	max	-1.262	15	251.948	2	-.593	12	0	2	-.001	15	.07	3
520			min	-29.611	1	-118.768	3	-41.58	1	0	3	-.025	1	-.138	2
521		14	max	-1.262	15	153.488	2	.517	3	0	2	-.001	12	.12	3
522			min	-29.611	1	-71.829	3	-19.168	1	0	3	-.041	1	-.245	2
523		15	max	-1.262	15	55.029	2	3.244	1	0	2	-.001	12	.146	3
524			min	-29.611	1	-24.891	3	-1.295	10	0	3	-.045	1	-.3	2
525		16	max	-1.262	15	22.048	3	25.656	1	0	2	0	3	.146	3
526			min	-29.611	1	-43.43	2	1.093	15	0	3	-.037	1	-.303	2
527		17	max	-1.262	15	68.987	3	48.069	1	0	2	.002	3	.122	3
528			min	-29.611	1	-141.89	2	1.995	15	0	3	-.018	1	-.254	2
529		18	max	-1.262	15	115.925	3	70.481	1	0	2	.014	1	.074	3
530			min	-29.611	1	-240.349	2	2.897	15	0	3	-.001	10	-.153	2
531		19	max	-1.262	15	162.864	3	92.893	1	0	2	.057	1	0	2
532			min	-29.611	1	-338.809	2	3.799	15	0	3	.002	15	0	3
533	M15	1	max	0	1	.983	3	.102	3	0	1	0	1	0	1
534			min	-90.654	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.873	3	.102	3	0	1	0	1	0	1
536			min	-90.725	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.764	3	.102	3	0	1	0	1	0	1
538			min	-90.795	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.655	3	.102	3	0	1	0	1	0	1
540			min	-90.866	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.546	3	.102	3	0	1	0	1	0	1
542			min	-90.936	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.437	3	.102	3	0	1	0	1	0	1
544			min	-91.007	3	0	1	0	1	0	3	0	3	-.001	3
545		7	max	0	1	.328	3	.102	3	0	1	0	3	0	1
546			min	-91.077	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.218	3	.102	3	0	1	0	3	0	1
548			min	-91.148	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.109	3	.102	3	0	1	0	3	0	1
550			min	-91.218	3	0	1	0	1	0	3	0	1	-.001	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.102	3	0	1	0	3	0	1
552		min	-91.289	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.102	3	0	1	0	3	0	1
554		min	-91.359	3	-.109	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.102	3	0	1	0	3	0	1
556		min	-91.43	3	-.218	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.102	3	0	1	0	3	0	1
558		min	-91.5	3	-.328	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.102	3	0	1	0	3	0	1
560		min	-91.571	3	-.437	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.102	3	0	1	0	3	0	1
562		min	-91.641	3	-.546	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.102	3	0	1	0	3	0	1
564		min	-91.712	3	-.655	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.102	3	0	1	0	3	0	1
566		min	-91.782	3	-.764	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.102	3	0	1	0	3	0	1
568		min	-91.853	3	-.873	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.102	3	0	1	0	3	0	1
570		min	-91.923	3	-.983	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.681	4	.033	1	0	3	0	3	0	1
572		min	-90.651	3	0	2	-.042	3	0	1	0	1	0	1
573	2	max	0	2	1.495	4	.033	1	0	3	0	3	0	2
574		min	-90.581	3	0	2	-.042	3	0	1	0	1	0	4
575	3	max	0	2	1.308	4	.033	1	0	3	0	3	0	2
576		min	-90.51	3	0	2	-.042	3	0	1	0	1	0	4
577	4	max	0	2	1.121	4	.033	1	0	3	0	3	0	2
578		min	-90.44	3	0	2	-.042	3	0	1	0	1	-.001	4
579	5	max	0	2	.934	4	.033	1	0	3	0	3	0	2
580		min	-90.369	3	0	2	-.042	3	0	1	0	1	-.002	4
581	6	max	0	2	.747	4	.033	1	0	3	0	3	0	2
582		min	-90.299	3	0	2	-.042	3	0	1	0	1	-.002	4
583	7	max	0	2	.56	4	.033	1	0	3	0	3	0	2
584		min	-90.228	3	0	2	-.042	3	0	1	0	1	-.002	4
585	8	max	0	2	.374	4	.033	1	0	3	0	3	0	2
586		min	-90.158	3	0	2	-.042	3	0	1	0	1	-.002	4
587	9	max	0	2	.187	4	.033	1	0	3	0	3	0	2
588		min	-90.087	3	0	2	-.042	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.033	1	0	3	0	3	0	2
590		min	-90.017	3	0	1	-.042	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.033	1	0	3	0	3	0	2
592		min	-89.946	3	-.187	4	-.042	3	0	1	0	1	-.002	4
593	12	max	0	2	0	2	.033	1	0	3	0	3	0	2
594		min	-89.876	3	-.374	4	-.042	3	0	1	0	1	-.002	4
595	13	max	.06	13	0	2	.033	1	0	3	0	1	0	2
596		min	-89.805	3	-.56	4	-.042	3	0	1	0	3	-.002	4
597	14	max	.157	13	0	2	.033	1	0	3	0	1	0	2
598		min	-89.735	3	-.747	4	-.042	3	0	1	0	3	-.002	4
599	15	max	.254	13	0	2	.033	1	0	3	0	1	0	2
600		min	-89.664	3	-.934	4	-.042	3	0	1	0	3	-.002	4
601	16	max	.351	13	0	2	.033	1	0	3	0	1	0	2
602		min	-89.594	3	-1.121	4	-.042	3	0	1	0	3	-.001	4
603	17	max	.448	13	0	2	.033	1	0	3	0	1	0	2
604		min	-89.523	3	-1.308	4	-.042	3	0	1	0	3	0	4
605	18	max	.545	13	0	2	.033	1	0	3	0	1	0	2
606		min	-89.453	3	-1.495	4	-.042	3	0	1	0	3	0	4
607	19	max	.641	13	0	2	.033	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-89.382	3	-1.681	4	-.042	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.009	2	.005	1	-1.933e-5	15	NC	3	NC	2	
2			min	-.004	3	-.009	3	-.002	3	-4.562e-4	1	4187.093	2	7432.025	1	
3		2	max	.002	1	.009	2	.005	1	-1.849e-5	15	NC	3	NC	2	
4			min	-.003	3	-.009	3	-.002	3	-4.363e-4	1	4569.089	2	8001.506	1	
5		3	max	.002	1	.008	2	.005	1	-1.766e-5	15	NC	3	NC	2	
6			min	-.003	3	-.009	3	-.002	3	-4.163e-4	1	5023.276	2	8674.74	1	
7		4	max	.002	1	.007	2	.004	1	-1.682e-5	15	NC	1	NC	2	
8			min	-.003	3	-.008	3	-.001	3	-3.964e-4	1	5567.037	2	9476.474	1	
9		5	max	.002	1	.006	2	.004	1	-1.599e-5	15	NC	1	NC	1	
10			min	-.003	3	-.008	3	-.001	3	-3.764e-4	1	6223.528	2	NC	1	
11		6	max	.002	1	.006	2	.003	1	-1.515e-5	15	NC	1	NC	1	
12			min	-.003	3	-.008	3	-.001	3	-3.564e-4	1	7024.098	2	NC	1	
13		7	max	.001	1	.005	2	.003	1	-1.432e-5	15	NC	1	NC	1	
14			min	-.002	3	-.007	3	0	3	-3.365e-4	1	8011.998	2	NC	1	
15		8	max	.001	1	.004	2	.003	1	-1.348e-5	15	NC	1	NC	1	
16			min	-.002	3	-.007	3	0	3	-3.165e-4	1	9248.232	2	NC	1	
17		9	max	.001	1	.004	2	.002	1	-1.265e-5	15	NC	1	NC	1	
18			min	-.002	3	-.006	3	0	3	-2.965e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.002	1	-1.181e-5	15	NC	1	NC	1	
20			min	-.002	3	-.006	3	0	3	-2.766e-4	1	NC	1	NC	1	
21		11	max	0	1	.003	2	.002	1	-1.098e-5	15	NC	1	NC	1	
22			min	-.002	3	-.005	3	0	3	-2.566e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.001	1	-1.004e-5	10	NC	1	NC	1	
24			min	-.001	3	-.005	3	0	3	-2.367e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.001	1	-9.053e-6	10	NC	1	NC	1	
26			min	-.001	3	-.004	3	0	3	-2.167e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	-8.069e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-1.967e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-7.085e-6	10	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-1.768e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-6.101e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-1.568e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-5.118e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-1.368e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-4.134e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-1.169e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-3.15e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-9.692e-5	1	NC	1	NC	1	
39		M3	1	max	0	1	0	1	0	1	4.576e-5	1	NC	1	NC	1
40				min	0	1	0	1	0	1	1.494e-6	10	NC	1	NC	1
41	2		max	0	3	0	2	0	10	5.745e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	2.193e-6	10	NC	1	NC	1	
43	3		max	0	3	0	2	0	10	6.914e-5	1	NC	1	NC	1	
44			min	0	2	-.002	3	0	1	2.892e-6	10	NC	1	NC	1	
45	4		max	0	3	0	2	0	3	8.083e-5	1	NC	1	NC	1	
46			min	0	2	-.003	3	0	1	3.455e-6	15	NC	1	NC	1	
47	5		max	0	3	0	2	0	3	9.252e-5	1	NC	1	NC	1	
48			min	0	2	-.003	3	0	1	3.932e-6	15	NC	1	NC	1	
49		6	max	0	3	0	2	0	3	1.042e-4	1	NC	1	NC	1	
50			min	0	2	-.004	3	0	1	4.408e-6	15	NC	1	NC	1	
51		7	max	0	3	0	2	0	3	1.159e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	1	4.884e-6	15	NC	1	NC	1
53		8	max	0	3	0	2	0	3	1.276e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	5.36e-6	15	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	1.393e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	5.836e-6	15	NC	1	NC	1
57		10	max	.001	3	.002	2	0	1	1.51e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	15	6.312e-6	15	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	1.627e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	15	6.789e-6	15	NC	1	NC	1
61		12	max	.001	3	.003	2	0	1	1.744e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	15	7.265e-6	15	NC	1	NC	1
63		13	max	.001	3	.003	2	.001	1	1.861e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	15	7.741e-6	15	NC	1	NC	1
65		14	max	.001	3	.004	2	.001	1	1.977e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	15	8.217e-6	15	NC	1	NC	1
67		15	max	.002	3	.005	2	.002	1	2.094e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	15	8.693e-6	15	8998.4	2	NC	1
69		16	max	.002	3	.006	2	.002	1	2.211e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	15	9.169e-6	15	7621.071	2	NC	1
71		17	max	.002	3	.007	2	.002	1	2.328e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	15	9.646e-6	15	6555.983	2	NC	1
73		18	max	.002	3	.008	2	.003	1	2.445e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	15	1.012e-5	15	5722.961	2	NC	1
75		19	max	.002	3	.009	2	.003	1	2.562e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	15	1.06e-5	15	5065.618	2	NC	1
77	M4	1	max	.001	1	.011	2	0	15	-1.494e-5	12	NC	1	NC	2
78			min	0	12	-.009	3	-.002	1	-3.598e-4	1	NC	1	8239.628	1
79		2	max	.001	1	.01	2	0	15	-1.494e-5	12	NC	1	NC	2
80			min	0	12	-.009	3	-.002	1	-3.598e-4	1	NC	1	8988.782	1
81		3	max	.001	1	.01	2	0	15	-1.494e-5	12	NC	1	NC	2
82			min	0	12	-.008	3	-.002	1	-3.598e-4	1	NC	1	9880.39	1
83		4	max	.001	1	.009	2	0	15	-1.494e-5	12	NC	1	NC	1
84			min	0	12	-.008	3	-.002	1	-3.598e-4	1	NC	1	NC	1
85		5	max	.001	1	.008	2	0	15	-1.494e-5	12	NC	1	NC	1
86			min	0	12	-.007	3	-.002	1	-3.598e-4	1	NC	1	NC	1
87		6	max	.001	1	.008	2	0	15	-1.494e-5	12	NC	1	NC	1
88			min	0	12	-.007	3	-.001	1	-3.598e-4	1	NC	1	NC	1
89		7	max	0	1	.007	2	0	15	-1.494e-5	12	NC	1	NC	1
90			min	0	12	-.006	3	-.001	1	-3.598e-4	1	NC	1	NC	1
91		8	max	0	1	.007	2	0	15	-1.494e-5	12	NC	1	NC	1
92			min	0	12	-.006	3	-.001	1	-3.598e-4	1	NC	1	NC	1
93		9	max	0	1	.006	2	0	15	-1.494e-5	12	NC	1	NC	1
94			min	0	12	-.005	3	0	1	-3.598e-4	1	NC	1	NC	1
95		10	max	0	1	.005	2	0	15	-1.494e-5	12	NC	1	NC	1
96			min	0	12	-.005	3	0	1	-3.598e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	15	-1.494e-5	12	NC	1	NC	1
98			min	0	12	-.004	3	0	1	-3.598e-4	1	NC	1	NC	1
99		12	max	0	1	.004	2	0	15	-1.494e-5	12	NC	1	NC	1
100			min	0	12	-.004	3	0	1	-3.598e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	15	-1.494e-5	12	NC	1	NC	1
102			min	0	12	-.003	3	0	1	-3.598e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	15	-1.494e-5	12	NC	1	NC	1
104			min	0	12	-.003	3	0	1	-3.598e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-1.494e-5	12	NC	1	NC	1
106			min	0	12	-.002	3	0	1	-3.598e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	15	-1.494e-5	12	NC	1	NC	1
108			min	0	12	-.002	3	0	1	-3.598e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-1.494e-5	12	NC	1	NC	1
110			min	0	12	-.001	3	0	1	-3.598e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.494e-5	12	NC	1	NC	1
112			min	0	12	0	3	0	1	-3.598e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.494e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-3.598e-4	1	NC	1	NC	1
115	M6	1	max	.007	1	.032	2	.002	1	4.398e-4	3	NC	3	NC	1
116			min	-.011	3	-.029	3	-.006	3	-7.861e-8	2	1249.082	2	6956.694	3
117		2	max	.006	1	.03	2	.002	1	4.262e-4	3	NC	3	NC	1
118			min	-.011	3	-.028	3	-.005	3	-7.414e-8	2	1336.683	2	7387.216	3
119		3	max	.006	1	.027	2	.002	1	4.125e-4	3	NC	3	NC	1
120			min	-.01	3	-.026	3	-.005	3	-1.903e-6	1	1437.08	2	7898.388	3
121		4	max	.006	1	.025	2	.001	1	3.989e-4	3	NC	3	NC	1
122			min	-.009	3	-.025	3	-.005	3	-4.565e-6	1	1552.837	2	8507.429	3
123		5	max	.005	1	.023	2	.001	1	3.853e-4	3	NC	3	NC	1
124			min	-.009	3	-.023	3	-.004	3	-7.227e-6	1	1687.256	2	9237.18	3
125		6	max	.005	1	.021	2	.001	1	3.716e-4	3	NC	3	NC	1
126			min	-.008	3	-.021	3	-.004	3	-9.89e-6	1	1844.664	2	NC	1
127		7	max	.005	1	.019	2	.001	1	3.58e-4	3	NC	3	NC	1
128			min	-.007	3	-.02	3	-.004	3	-1.255e-5	1	2030.83	2	NC	1
129		8	max	.004	1	.017	2	0	1	3.444e-4	3	NC	3	NC	1
130			min	-.007	3	-.018	3	-.003	3	-1.521e-5	1	2253.628	2	NC	1
131		9	max	.004	1	.016	2	0	1	3.307e-4	3	NC	3	NC	1
132			min	-.006	3	-.017	3	-.003	3	-1.788e-5	1	2524.081	2	NC	1
133		10	max	.003	1	.014	2	0	1	3.171e-4	3	NC	3	NC	1
134			min	-.006	3	-.015	3	-.002	3	-2.054e-5	1	2858.114	2	NC	1
135		11	max	.003	1	.012	2	0	1	3.035e-4	3	NC	3	NC	1
136			min	-.005	3	-.013	3	-.002	3	-2.32e-5	1	3279.605	2	NC	1
137		12	max	.003	1	.01	2	0	1	2.898e-4	3	NC	3	NC	1
138			min	-.004	3	-.012	3	-.002	3	-2.586e-5	1	3826.07	2	NC	1
139		13	max	.002	1	.009	2	0	1	2.762e-4	3	NC	3	NC	1
140			min	-.004	3	-.01	3	-.001	3	-2.853e-5	1	4560.022	2	NC	1
141		14	max	.002	1	.007	2	0	1	2.626e-4	3	NC	3	NC	1
142			min	-.003	3	-.009	3	-.001	3	-3.119e-5	1	5593.951	2	NC	1
143		15	max	.002	1	.006	2	0	1	2.489e-4	3	NC	1	NC	1
144			min	-.002	3	-.007	3	0	3	-3.385e-5	1	7152.792	2	NC	1
145		16	max	.001	1	.004	2	0	1	2.353e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-3.651e-5	1	9761.316	2	NC	1
147		17	max	0	1	.003	2	0	1	2.217e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-3.918e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.08e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-4.184e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.944e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.45e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.086e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.122e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.819e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.816e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.553e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.51e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	1.287e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.203e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	1.021e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	0	2	8402.474	2	NC	1
163		6	max	.002	3	.007	2	.002	3	2.409e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	0	2	6733.343	2	NC	1
165		7	max	.002	3	.008	2	.002	3	4.716e-5	3	NC	3	NC	1











Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.378e-4	3	NC	1	NC	1
338			min	0	12	-.001	3	0	10	1.319e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.378e-4	3	NC	1	NC	1
340			min	0	12	0	3	0	10	1.319e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.378e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	1.319e-5	15	NC	1	NC	1
343	M1	1	max	.008	3	.025	3	.003	3	7.999e-3	2	NC	1	NC	1
344			min	-.009	2	-.021	2	-.002	1	-1.154e-2	3	NC	1	NC	1
345		2	max	.008	3	.015	3	.002	3	3.931e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.004	1	-5.697e-3	3	4685.091	3	NC	1
347		3	max	.008	3	.005	3	.002	3	3.738e-5	3	NC	4	NC	1
348			min	-.009	2	-.004	2	-.005	1	-2.641e-4	1	2427.145	3	NC	1
349		4	max	.008	3	.004	2	.001	3	3.909e-5	3	NC	4	NC	1
350			min	-.009	2	-.003	3	-.006	1	-2.251e-4	1	1729.828	3	NC	1
351		5	max	.008	3	.01	2	.001	3	4.08e-5	3	NC	4	NC	1
352			min	-.009	2	-.009	3	-.006	1	-1.861e-4	1	1397.402	3	NC	1
353		6	max	.008	3	.016	2	0	3	4.251e-5	3	NC	4	NC	1
354			min	-.009	2	-.015	3	-.006	1	-1.471e-4	1	1208.644	2	NC	1
355		7	max	.008	3	.02	2	0	3	4.422e-5	3	NC	4	NC	1
356			min	-.009	2	-.019	3	-.005	1	-1.08e-4	1	1075.684	2	NC	1
357		8	max	.008	3	.023	2	0	3	4.593e-5	3	NC	4	NC	1
358			min	-.009	2	-.022	3	-.004	1	-6.901e-5	1	992.044	2	NC	1
359		9	max	.008	3	.025	2	0	3	4.763e-5	3	NC	4	NC	1
360			min	-.009	2	-.023	3	-.003	1	-3.07e-5	9	942.142	2	NC	1
361		10	max	.008	3	.026	2	0	3	4.934e-5	3	NC	4	NC	1
362			min	-.009	2	-.024	3	-.002	1	-3.23e-6	9	918.516	2	NC	1
363		11	max	.008	3	.026	2	0	3	5.105e-5	3	NC	4	NC	1
364			min	-.009	2	-.023	3	0	9	1.692e-6	15	918.439	2	NC	1
365		12	max	.008	3	.024	2	.001	1	8.71e-5	1	NC	4	NC	1
366			min	-.009	2	-.021	3	0	15	3.376e-6	15	942.896	2	NC	1
367		13	max	.008	3	.021	2	.002	1	1.261e-4	1	NC	4	NC	1
368			min	-.009	2	-.018	3	0	15	5.06e-6	15	997.159	2	NC	1
369		14	max	.008	3	.017	2	.003	1	1.652e-4	1	NC	4	NC	1
370			min	-.009	2	-.014	3	0	15	6.744e-6	15	1093.474	2	NC	1
371		15	max	.008	3	.011	2	.003	1	2.042e-4	1	NC	4	NC	1
372			min	-.009	2	-.009	3	0	15	8.428e-6	15	1258.798	2	NC	1
373		16	max	.008	3	.003	2	.003	1	2.316e-4	1	NC	4	NC	1
374			min	-.009	2	-.003	3	0	15	9.607e-6	15	1559.259	2	NC	1
375		17	max	.008	3	.005	3	.002	1	5.544e-5	3	NC	4	NC	1
376			min	-.009	2	-.006	2	0	15	-2.553e-5	9	2207.903	2	NC	1
377		18	max	.008	3	.012	3	0	1	5.738e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	15	-2.88e-3	3	4278.448	2	NC	1
379		19	max	.008	3	.021	3	0	3	1.156e-2	2	NC	1	NC	1
380			min	-.009	2	-.028	2	-.001	1	-5.871e-3	3	NC	1	NC	1
381	M5	1	max	.026	3	.08	3	.003	3	5.229e-6	3	NC	1	NC	1
382			min	-.029	2	-.069	2	-.002	1	0	15	NC	1	NC	1
383		2	max	.026	3	.047	3	.004	3	1.226e-4	3	NC	4	NC	1
384			min	-.029	2	-.04	2	-.002	1	-3.222e-5	1	1467.753	3	NC	1
385		3	max	.026	3	.017	3	.006	3	2.377e-4	3	NC	5	NC	1
386			min	-.029	2	-.013	2	-.002	1	-6.39e-5	1	760.688	3	NC	1
387		4	max	.026	3	.012	2	.007	3	2.299e-4	3	NC	5	NC	1
388			min	-.029	2	-.009	3	-.002	1	-6.065e-5	1	542.675	3	NC	1
389		5	max	.026	3	.033	2	.007	3	2.221e-4	3	NC	5	NC	1
390			min	-.029	2	-.03	3	-.002	1	-5.74e-5	1	437.347	2	NC	1
391		6	max	.026	3	.051	2	.008	3	2.143e-4	3	NC	5	NC	1
392			min	-.029	2	-.047	3	-.002	1	-5.416e-5	1	371.018	2	9271.772	3
393		7	max	.026	3	.065	2	.008	3	2.066e-4	3	NC	5	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.008	3	.005	3	0	10	6.785e-5	3	NC	4	NC	1
452			min	-.009	2	-.006	2	-.004	1	-1.437e-4	1	2208.596	2	NC	1
453		18	max	.008	3	.012	3	0	10	2.943e-3	3	NC	4	NC	1
454			min	-.009	2	-.017	2	-.003	1	-5.738e-3	2	4279.747	2	NC	1
455		19	max	.008	3	.021	3	0	3	5.869e-3	3	NC	1	NC	1
456			min	-.009	2	-.028	2	0	1	-1.156e-2	2	NC	1	NC	1
457	M13	1	max	.002	1	.025	3	.009	3	3.866e-3	3	NC	1	NC	1
458			min	-.003	3	-.021	2	-.009	2	-3.395e-3	2	NC	1	NC	1
459		2	max	.002	1	.095	3	.007	9	4.813e-3	3	NC	4	NC	1
460			min	-.003	3	-.07	2	-.006	2	-4.243e-3	2	1612.665	3	NC	1
461		3	max	.002	1	.154	3	.017	1	5.761e-3	3	NC	5	NC	2
462			min	-.003	3	-.111	2	-.005	10	-5.09e-3	2	880.648	3	5188.921	1
463		4	max	.002	1	.193	3	.027	1	6.709e-3	3	NC	5	NC	2
464			min	-.003	3	-.139	2	-.005	10	-5.938e-3	2	677.814	3	3644.198	1
465		5	max	.002	1	.208	3	.03	1	7.657e-3	3	NC	5	NC	2
466			min	-.003	3	-.15	2	-.007	10	-6.786e-3	2	622.94	3	3296.445	1
467		6	max	.002	1	.199	3	.026	1	8.605e-3	3	NC	5	NC	2
468			min	-.003	3	-.146	2	-.009	10	-7.633e-3	2	654.475	3	3721.222	1
469		7	max	.002	1	.171	3	.018	9	9.553e-3	3	NC	5	NC	2
470			min	-.003	3	-.128	2	-.012	2	-8.481e-3	2	779.675	3	5656.095	1
471		8	max	.002	1	.133	3	.02	3	1.05e-2	3	NC	5	NC	1
472			min	-.003	3	-.104	2	-.02	2	-9.329e-3	2	1057.891	3	9784.021	3
473		9	max	.002	1	.097	3	.023	3	1.145e-2	3	NC	4	NC	1
474			min	-.003	3	-.08	2	-.026	2	-1.018e-2	2	1590.261	3	6470.477	2
475		10	max	.002	1	.08	3	.026	3	1.24e-2	3	NC	4	NC	4
476			min	-.003	3	-.069	2	-.029	2	-1.102e-2	2	2071.217	3	5524.65	2
477		11	max	.002	1	.097	3	.028	3	1.145e-2	3	NC	4	NC	1
478			min	-.003	3	-.08	2	-.026	2	-1.018e-2	2	1590.259	3	5763.46	3
479		12	max	.002	1	.133	3	.029	3	1.05e-2	3	NC	5	NC	1
480			min	-.003	3	-.104	2	-.02	2	-9.329e-3	2	1057.89	3	5556.896	3
481		13	max	.002	1	.171	3	.028	3	9.558e-3	3	NC	5	NC	2
482			min	-.003	3	-.128	2	-.012	2	-8.481e-3	2	779.674	3	5630.557	1
483		14	max	.002	1	.199	3	.026	3	8.612e-3	3	NC	5	NC	2
484			min	-.003	3	-.146	2	-.009	10	-7.633e-3	2	654.474	3	3717.853	1
485		15	max	.002	1	.208	3	.03	1	7.666e-3	3	NC	5	NC	2
486			min	-.003	3	-.15	2	-.007	10	-6.786e-3	2	622.94	3	3300.841	1
487		16	max	.002	1	.193	3	.027	1	6.72e-3	3	NC	5	NC	2
488			min	-.003	3	-.139	2	-.005	10	-5.938e-3	2	677.813	3	3656.809	1
489		17	max	.002	1	.155	3	.017	1	5.774e-3	3	NC	5	NC	2
490			min	-.003	3	-.111	2	-.005	10	-5.09e-3	2	880.648	3	5221.14	1
491		18	max	.002	1	.096	3	.011	3	4.828e-3	3	NC	4	NC	1
492			min	-.003	3	-.07	2	-.006	2	-4.243e-3	2	1612.664	3	NC	1
493		19	max	.002	1	.025	3	.008	3	3.882e-3	3	NC	1	NC	1
494			min	-.003	3	-.021	2	-.009	2	-3.395e-3	2	NC	1	NC	1
495	M16	1	max	0	1	.021	3	.008	3	4.297e-3	2	NC	1	NC	1
496			min	0	3	-.028	2	-.009	2	-3.136e-3	3	NC	1	NC	1
497		2	max	0	1	.057	3	.011	3	5.377e-3	2	NC	4	NC	1
498			min	0	3	-.099	2	-.006	2	-3.88e-3	3	1608.802	2	NC	1
499		3	max	0	1	.087	3	.017	1	6.457e-3	2	NC	5	NC	2
500			min	0	3	-.158	2	-.005	10	-4.623e-3	3	876.892	2	5196.77	1
501		4	max	0	1	.108	3	.027	1	7.537e-3	2	NC	5	NC	2
502			min	0	3	-.198	2	-.005	10	-5.367e-3	3	672.668	2	3649.699	1
503		5	max	0	1	.118	3	.03	1	8.617e-3	2	NC	5	NC	2
504			min	0	3	-.214	2	-.006	10	-6.111e-3	3	614.85	2	3302.181	1
505		6	max	0	1	.116	3	.026	1	9.697e-3	2	NC	5	NC	2
506			min	0	3	-.206	2	-.009	10	-6.854e-3	3	640.248	2	3730.049	1
507		7	max	0	1	.105	3	.026	3	1.078e-2	2	NC	5	NC	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.18	2	-.012	2	-7.598e-3	3	751.232	2	5680.04	1
509	8	max	0	1	.088	3	.027	3	1.186e-2	2	NC	5	NC	1
510		min	0	3	-.143	2	-.02	2	-8.342e-3	3	991.844	2	6164.072	3
511	9	max	0	1	.072	3	.026	3	1.294e-2	2	NC	4	NC	1
512		min	0	3	-.108	2	-.026	2	-9.085e-3	3	1422.2	2	6294.385	3
513	10	max	0	1	.065	3	.025	3	1.402e-2	2	NC	4	NC	4
514		min	0	3	-.092	2	-.029	2	-9.829e-3	3	1779.775	2	5550.703	2
515	11	max	0	1	.072	3	.024	3	1.294e-2	2	NC	4	NC	1
516		min	0	3	-.108	2	-.026	2	-9.084e-3	3	1422.2	2	6504.65	2
517	12	max	0	1	.088	3	.022	3	1.186e-2	2	NC	5	NC	1
518		min	0	3	-.143	2	-.02	2	-8.338e-3	3	991.844	2	8025.727	3
519	13	max	.001	1	.105	3	.021	3	1.078e-2	2	NC	5	NC	2
520		min	0	3	-.18	2	-.012	2	-7.593e-3	3	751.232	2	5672.844	1
521	14	max	.001	1	.116	3	.026	1	9.697e-3	2	NC	5	NC	2
522		min	0	3	-.206	2	-.009	10	-6.847e-3	3	640.248	2	3735.549	1
523	15	max	.001	1	.117	3	.03	1	8.618e-3	2	NC	5	NC	2
524		min	0	3	-.214	2	-.006	10	-6.102e-3	3	614.85	2	3313.534	1
525	16	max	.001	1	.108	3	.026	1	7.538e-3	2	NC	5	NC	2
526		min	0	3	-.198	2	-.005	10	-5.357e-3	3	672.668	2	3669.956	1
527	17	max	.001	1	.087	3	.017	1	6.458e-3	2	NC	5	NC	2
528		min	0	3	-.158	2	-.005	10	-4.611e-3	3	876.892	2	5241.064	1
529	18	max	.001	1	.057	3	.01	3	5.379e-3	2	NC	4	NC	1
530		min	0	3	-.099	2	-.006	2	-3.866e-3	3	1608.802	2	NC	1
531	19	max	.001	1	.021	3	.008	3	4.299e-3	2	NC	1	NC	1
532		min	0	3	-.028	2	-.009	2	-3.121e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.836e-5	2	NC	1	NC	1
535	2	max	0	3	-.001	15	0	1	8.534e-4	3	NC	1	NC	1
536		min	0	2	-.005	4	0	3	-5.077e-4	2	NC	1	NC	1
537	3	max	0	3	-.002	15	.003	1	1.307e-3	3	NC	3	NC	1
538		min	0	2	-.009	4	-.003	3	-9.57e-4	2	7777.64	4	9908.837	3
539	4	max	0	3	-.003	15	.006	1	1.76e-3	3	NC	5	NC	4
540		min	0	2	-.013	4	-.007	3	-1.406e-3	2	5335.915	4	5498.763	3
541	5	max	0	3	-.004	15	.01	2	2.214e-3	3	NC	5	NC	4
542		min	0	2	-.017	4	-.012	3	-1.856e-3	2	4163.669	4	3622.965	3
543	6	max	0	3	-.005	15	.014	2	2.667e-3	3	NC	15	NC	4
544		min	-.001	2	-.02	4	-.017	3	-2.305e-3	2	3504.167	4	2645.023	3
545	7	max	0	3	-.005	15	.019	2	3.121e-3	3	NC	15	NC	4
546		min	-.001	2	-.023	4	-.023	3	-2.754e-3	2	3107.564	4	2071.731	3
547	8	max	0	3	-.006	15	.023	2	3.574e-3	3	NC	15	NC	4
548		min	-.001	2	-.024	4	-.028	3	-3.203e-3	2	2869.543	4	1710.716	3
549	9	max	0	3	-.006	15	.027	2	4.028e-3	3	NC	15	NC	4
550		min	-.002	2	-.026	4	-.033	3	-3.653e-3	2	2741.426	4	1474.209	3
551	10	max	0	3	-.006	15	.03	2	4.481e-3	3	NC	15	NC	4
552		min	-.002	2	-.026	4	-.037	3	-4.102e-3	2	2700.895	4	1318.041	3
553	11	max	0	3	-.006	15	.032	2	4.935e-3	3	NC	15	NC	5
554		min	-.002	2	-.026	4	-.039	3	-4.551e-3	2	2741.426	4	1219.007	3
555	12	max	.001	3	-.006	15	.032	2	5.388e-3	3	NC	15	NC	5
556		min	-.002	2	-.025	4	-.04	3	-5.001e-3	2	2869.543	4	1165.61	3
557	13	max	.001	3	-.005	15	.031	1	5.841e-3	3	NC	15	NC	5
558		min	-.002	2	-.023	4	-.039	3	-5.45e-3	2	3107.564	4	1154.581	3
559	14	max	.001	3	-.005	15	.028	1	6.295e-3	3	NC	15	NC	4
560		min	-.003	2	-.02	4	-.035	3	-5.899e-3	2	3504.167	4	1191.042	3
561	15	max	.001	3	-.004	15	.023	1	6.748e-3	3	NC	5	NC	4
562		min	-.003	2	-.017	4	-.029	3	-6.349e-3	2	4163.669	4	1293.574	3
563	16	max	.001	3	-.003	15	.016	1	7.202e-3	3	NC	5	NC	4
564		min	-.003	2	-.014	4	-.019	3	-6.798e-3	2	5335.915	4	1512.532	3

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.001	3	-.001	2	.007	1	7.655e-3	3	NC	3	NC	4
566			min	-.003	2	-.01	4	-.006	3	-7.247e-3	2	7777.64	4	2005.825	3
567		18	max	.002	3	.002	2	.01	3	8.109e-3	3	NC	1	NC	4
568			min	-.004	2	-.005	4	-.013	2	-7.696e-3	2	NC	1	3572.166	3
569		19	max	.002	3	.006	2	.032	3	8.562e-3	3	NC	1	NC	1
570			min	-.004	2	-.002	9	-.031	2	-8.146e-3	2	NC	1	NC	1
571	M16A	1	max	0	10	.001	2	.01	3	2.46e-3	3	NC	1	NC	1
572			min	-.002	3	0	3	-.01	2	-2.422e-3	2	NC	1	NC	1
573		2	max	0	10	-.001	15	.002	3	2.365e-3	3	NC	1	NC	1
574			min	-.002	3	-.005	4	-.004	2	-2.314e-3	2	NC	1	NC	1
575		3	max	0	10	-.002	15	.005	1	2.27e-3	3	NC	3	NC	4
576			min	-.001	3	-.009	4	-.004	3	-2.205e-3	2	7777.64	4	5706.896	3
577		4	max	0	10	-.003	15	.008	1	2.176e-3	3	NC	5	NC	4
578			min	-.001	3	-.013	4	-.008	3	-2.097e-3	2	5335.915	4	4345.748	3
579		5	max	0	10	-.004	15	.011	1	2.081e-3	3	NC	5	NC	4
580			min	-.001	3	-.017	4	-.011	3	-1.988e-3	2	4163.669	4	3758.387	3
581		6	max	0	10	-.005	15	.012	1	1.986e-3	3	NC	15	NC	4
582			min	-.001	3	-.02	4	-.013	3	-1.88e-3	2	3504.167	4	3505.265	3
583		7	max	0	10	-.005	15	.013	1	1.891e-3	3	NC	15	NC	4
584			min	-.001	3	-.023	4	-.014	3	-1.771e-3	2	3107.564	4	3449.176	3
585		8	max	0	10	-.006	15	.013	1	1.796e-3	3	NC	15	NC	4
586			min	-.001	3	-.024	4	-.014	3	-1.663e-3	2	2869.543	4	3544.055	3
587		9	max	0	10	-.006	15	.012	1	1.702e-3	3	NC	15	NC	4
588			min	0	3	-.026	4	-.013	3	-1.555e-3	2	2741.426	4	3785.385	3
589		10	max	0	10	-.006	15	.011	1	1.607e-3	3	NC	15	NC	4
590			min	0	3	-.026	4	-.012	3	-1.446e-3	2	2700.895	4	4199.357	3
591		11	max	0	10	-.006	15	.009	1	1.512e-3	3	NC	15	NC	4
592			min	0	3	-.026	4	-.01	3	-1.338e-3	2	2741.426	4	4849.433	3
593		12	max	0	10	-.006	15	.008	1	1.417e-3	3	NC	15	NC	4
594			min	0	3	-.024	4	-.008	3	-1.229e-3	2	2869.543	4	5862.299	3
595		13	max	0	10	-.005	15	.006	1	1.323e-3	3	NC	15	NC	2
596			min	0	3	-.022	4	-.006	3	-1.121e-3	2	3107.564	4	7495.076	3
597		14	max	0	10	-.005	15	.004	1	1.228e-3	3	NC	15	NC	1
598			min	0	3	-.02	4	-.004	3	-1.012e-3	2	3504.167	4	NC	1
599		15	max	0	10	-.004	15	.003	1	1.133e-3	3	NC	5	NC	1
600			min	0	3	-.017	4	-.002	3	-9.038e-4	2	4163.669	4	NC	1
601		16	max	0	10	-.003	15	.001	9	1.038e-3	3	NC	5	NC	1
602			min	0	3	-.013	4	0	3	-7.953e-4	2	5335.915	4	NC	1
603		17	max	0	10	-.002	15	0	4	9.434e-4	3	NC	3	NC	1
604			min	0	3	-.009	4	0	2	-6.868e-4	2	7777.64	4	NC	1
605		18	max	0	10	-.001	15	0	3	8.487e-4	3	NC	1	NC	1
606			min	0	3	-.005	4	0	2	-5.784e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	7.539e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-4.699e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Software  
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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

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