

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

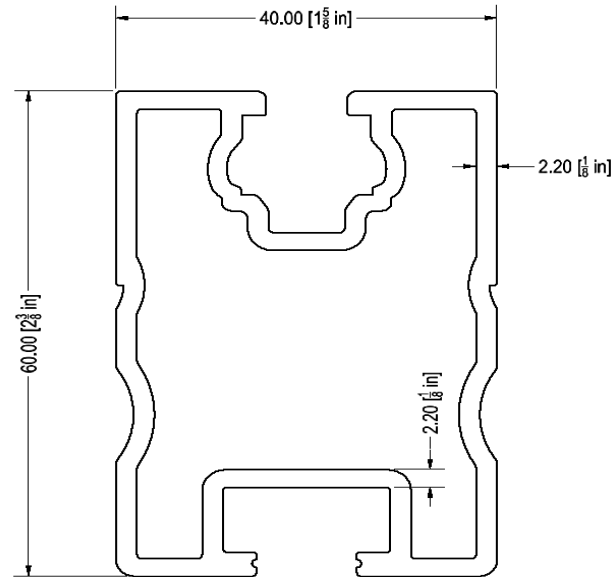
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

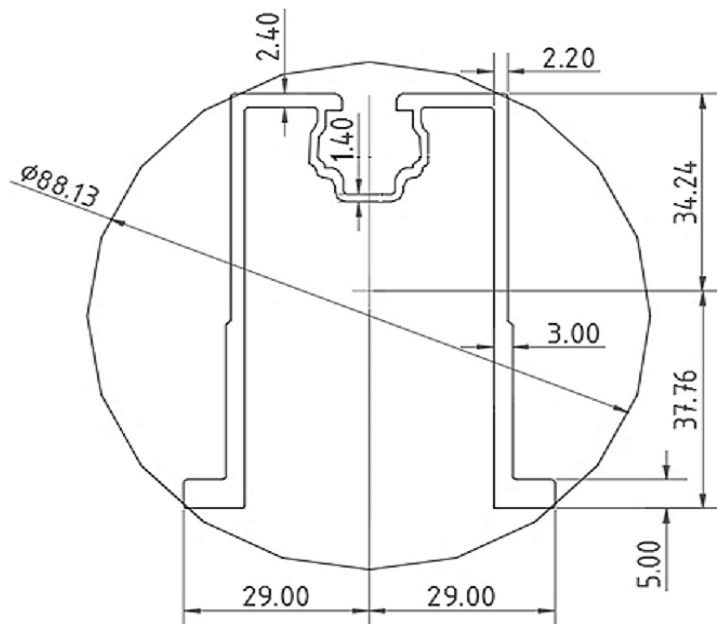
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42 in
ΦF_{ty} STRONG-AXIS =	29.99 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	-0.330 k-ft
M_z =	-0.015 k-ft
$M_{y \text{ allowable}}$ =	1.276 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	28%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

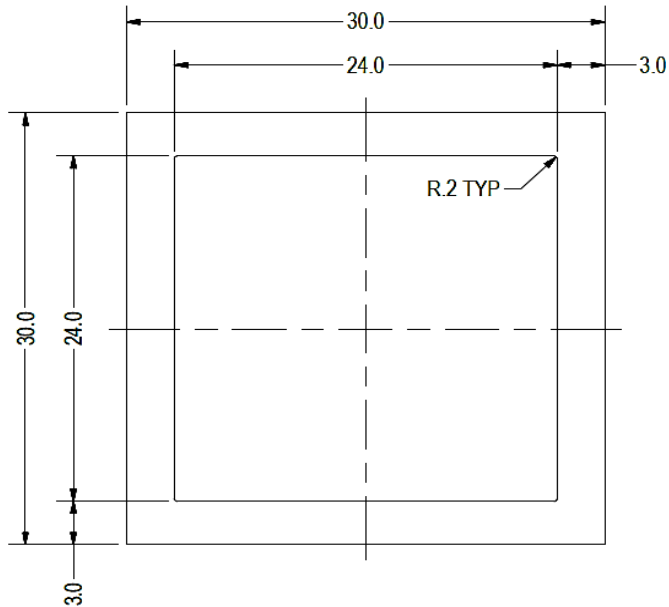
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.73 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.444 k-ft
M_z =	-0.020 k-ft
P_n =	0.148 k
$M_{y \text{ allowable}}$ =	1.459 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	35%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.023 k-ft
P_n =	0.129 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.426 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	11%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

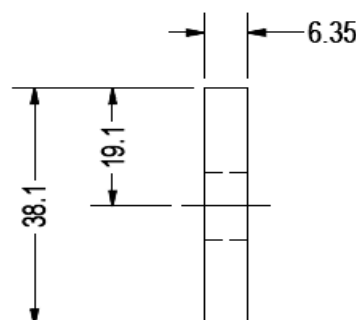
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.613 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	11%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.159 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

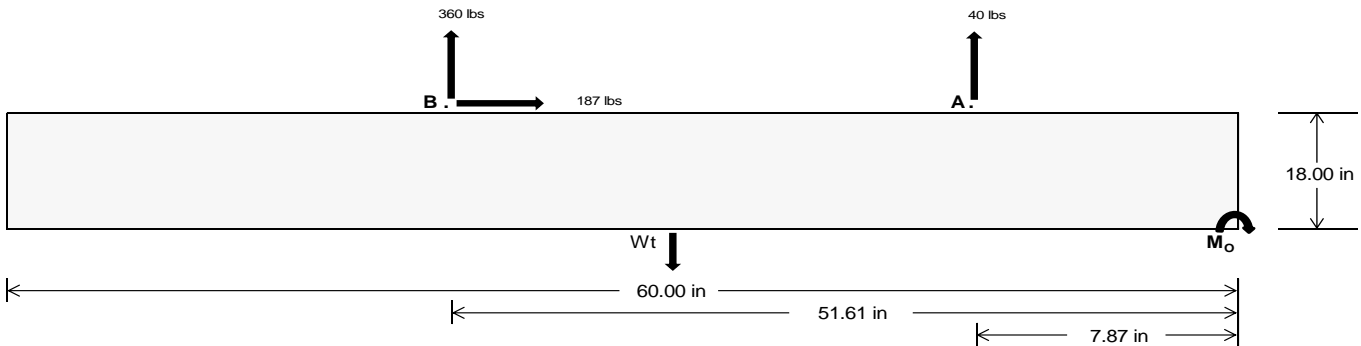
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	180.08	1562.90	k
Compressive Load =	994.88	1012.52	k
Lateral Load =	18.85	810.68	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 22267.3$ in-lbs
Resisting Force Required = 742.24 lbs
S.F. = 1.67
Weight Required = 1237.07 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 187.04 lbs
Friction = 0.4
Weight Required = 467.61 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 187.04 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	310 lbs	310 lbs	310 lbs	310 lbs	397 lbs	397 lbs	397 lbs	397 lbs	504 lbs	504 lbs	504 lbs	504 lbs	-81 lbs	-81 lbs	-81 lbs	-81 lbs
F_B	213 lbs	213 lbs	213 lbs	213 lbs	425 lbs	425 lbs	425 lbs	425 lbs	460 lbs	460 lbs	460 lbs	460 lbs	-720 lbs	-720 lbs	-720 lbs	-720 lbs
F_V	21 lbs	21 lbs	21 lbs	21 lbs	331 lbs	331 lbs	331 lbs	331 lbs	263 lbs	263 lbs	263 lbs	263 lbs	-374 lbs	-374 lbs	-374 lbs	-374 lbs
P_{total}	2336 lbs	2427 lbs	2518 lbs	2608 lbs	2634 lbs	2725 lbs	2816 lbs	2906 lbs	2777 lbs	2867 lbs	2958 lbs	3048 lbs	286 lbs	341 lbs	395 lbs	450 lbs
M	220 lbs-ft	220 lbs-ft	220 lbs-ft	220 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	495 lbs-ft	495 lbs-ft	495 lbs-ft	495 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft
e	0.09 ft	0.09 ft	0.09 ft	0.08 ft	0.18 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.05 ft	1.72 ft	1.48 ft	1.30 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	248.7 psf	247.2 psf	245.8 psf	244.6 psf	249.6 psf	248.1 psf	246.7 psf	245.4 psf	261.9 psf	259.8 psf	257.9 psf	256.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	312.0 psf	307.5 psf	303.4 psf	299.7 psf	382.6 psf	374.8 psf	367.6 psf	361.1 psf	404.4 psf	395.5 psf	387.4 psf	380.1 psf	252.9 psf	166.6 psf	141.4 psf	130.8 psf

Maximum Bearing Pressure = 404 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

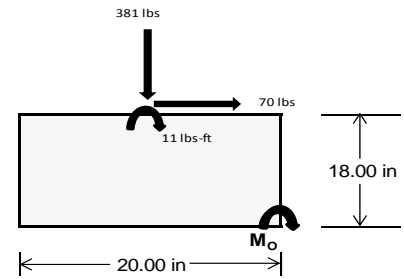
Overturning Check

$M_o = 201.7 \text{ ft-lbs}$
 Resisting Force Required = 242.01 lbs
 S.F. = 1.67
 Weight Required = 403.35 lbs
 Minimum Width = 20 in in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	109 lbs	44 lbs	50 lbs	201 lbs	381 lbs	156 lbs	76 lbs	-35 lbs	18 lbs
F_v	11 lbs	93 lbs	11 lbs	8 lbs	70 lbs	8 lbs	11 lbs	93 lbs	11 lbs
P_{total}	2353 lbs	2288 lbs	2294 lbs	2337 lbs	2517 lbs	2292 lbs	732 lbs	621 lbs	675 lbs
M	31 lbs-ft	154 lbs-ft	32 lbs-ft	22 lbs-ft	116 lbs-ft	25 lbs-ft	31 lbs-ft	154 lbs-ft	32 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.25 ft	0.05 ft
$L/6$	0.28 ft	1.53 ft	1.64 ft	1.65 ft	1.57 ft	1.65 ft	1.58 ft	1.17 ft	1.57 ft
f_{min}	269.1 sqft	208.0 sqft	261.6 sqft	270.9 sqft	252.1 sqft	264.3 sqft	74.5 sqft	8.0 sqft	67.3 sqft
f_{max}	295.7 psf	341.1 psf	288.9 psf	290.0 psf	351.9 psf	285.6 psf	101.2 psf	141.1 psf	94.6 psf



Maximum Bearing Pressure = 352 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

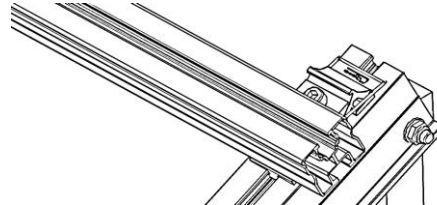
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.834 k
Allowable Uplift =	1.214 k
Utilization =	<u>69%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.044 k
Allowable Uplift =	1.116 k
Utilization =	<u>94%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.765 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

Diagonal Strut

Maximum Axial Load =	0.426 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>7%</u>



Rear Strut

Maximum Axial Load =	1.033 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Bracing

Maximum Axial Load =	0.159 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.047 in
	<u>0.047 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 23.9$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 30$$

$$Cc = 30$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 250988 \text{ mm}^4$$

$$0.603 \text{ in}^4$$

$$y = 30 \text{ mm}$$

$$S_x = 0.511 \text{ in}^3$$

$$M_{\max} St = 1.276 \text{ k-ft}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20$$

$$Cc = 20$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.5 \text{ ksi}$$

$$I_y = 120291 \text{ mm}^4$$

$$0.289 \text{ in}^4$$

$$x = 20 \text{ mm}$$

$$S_y = 0.367 \text{ in}^3$$

$$M_{\max} Wk = 0.871 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 7.4$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

$$b/t = 23.9$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi c [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.47 \text{ ksi}$$

$$A = 578.06 \text{ mm}^2$$

$$0.90 \text{ in}^2$$

$$P_{\max} = 25.51 \text{ kips}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &21.5728 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y F_{cy} \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.459 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.5514 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.7972 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 11.5927 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

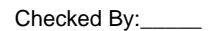
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 11.59 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.82 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 25° 160mph 30psf 3.5ft 7-10.rdb Page 20



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	200.459	2	.034	2	.068	1	0	10	0	4	0	15
30		min	-347.603	3	-.029	3	-.536	5	0	4	0	3	0	6
31	16	max	200.576	2	-.002	2	.068	1	0	10	0	4	0	15
32		min	-347.515	3	-.056	3	-.642	5	0	4	0	3	0	6
33	17	max	200.692	2	-.022	15	.068	1	0	10	0	4	0	15
34		min	-347.428	3	-.089	4	-.747	5	0	4	0	3	0	6
35	18	max	200.809	2	-.033	15	.068	1	0	10	0	9	0	15
36		min	-347.341	3	-.135	4	-.853	5	0	4	0	3	0	6
37	19	max	200.925	2	-.044	15	.068	1	0	10	0	9	0	15
38		min	-347.254	3	-.181	4	-.958	5	0	4	0	3	0	6
39	M3	1	max	142.025	2	1.777	.01	10	0	5	0	4	0	6
40		min	-129.741	3	.417	15	-1.327	4	0	1	0	10	0	15
41	2	max	141.957	2	1.6	6	.01	10	0	5	0	1	0	2
42		min	-129.793	3	.375	15	-1.194	4	0	1	0	10	0	15
43	3	max	141.888	2	1.422	6	.01	10	0	5	0	1	0	2
44		min	-129.844	3	.333	15	-1.06	4	0	1	0	5	0	3
45	4	max	141.82	2	1.245	6	.01	10	0	5	0	1	0	15
46		min	-129.896	3	.292	15	-.926	4	0	1	0	5	0	4
47	5	max	141.751	2	1.068	6	.01	10	0	5	0	1	0	15
48		min	-129.947	3	.25	15	-.793	4	0	1	0	5	0	4
49	6	max	141.682	2	.891	6	.01	10	0	5	0	1	0	15
50		min	-129.999	3	.208	15	-.659	4	0	1	0	5	0	4
51	7	max	141.614	2	.714	6	.01	10	0	5	0	1	0	15
52		min	-130.05	3	.167	15	-.525	4	0	1	0	5	0	4
53	8	max	141.545	2	.536	6	.01	10	0	5	0	1	0	15
54		min	-130.101	3	.125	15	-.392	4	0	1	0	5	-.001	4
55	9	max	141.477	2	.359	6	.01	10	0	5	0	1	0	15
56		min	-130.153	3	.083	15	-.258	4	0	1	0	5	-.001	4
57	10	max	141.408	2	.182	6	.01	10	0	5	0	1	0	15
58		min	-130.204	3	.042	15	-.125	4	0	1	0	5	-.001	4
59	11	max	141.339	2	.029	2	.034	5	0	5	0	1	0	15
60		min	-130.256	3	-.021	3	-.103	1	0	1	0	5	-.001	4
61	12	max	141.271	2	-.042	15	.168	5	0	5	0	1	0	15
62		min	-130.307	3	-.173	4	-.103	1	0	1	0	5	-.001	4
63	13	max	141.202	2	-.083	15	.301	5	0	5	0	1	0	15
64		min	-130.359	3	-.35	4	-.103	1	0	1	0	5	-.001	4
65	14	max	141.133	2	-.125	15	.435	5	0	5	0	9	0	15
66		min	-130.41	3	-.527	4	-.103	1	0	1	0	5	-.001	4
67	15	max	141.065	2	-.167	15	.569	5	0	5	0	9	0	15
68		min	-130.462	3	-.704	4	-.103	1	0	1	0	5	0	4
69	16	max	140.996	2	-.208	15	.702	5	0	5	0	10	0	15
70		min	-130.513	3	-.881	4	-.103	1	0	1	0	4	0	4
71	17	max	140.928	2	-.25	15	.836	5	0	5	0	10	0	15
72		min	-130.565	3	-1.059	4	-.103	1	0	1	0	4	0	4
73	18	max	140.859	2	-.292	15	.969	5	0	5	0	10	0	15
74		min	-130.616	3	-1.236	4	-.103	1	0	1	0	4	0	4
75	19	max	140.79	2	-.333	15	1.103	5	0	5	0	5	0	1
76		min	-130.667	3	-1.413	4	-.103	1	0	1	0	1	0	1
77	M4	1	max	264.412	1	0	.048	10	0	1	0	5	0	1
78		min	-30.771	3	0	1	-13.31	4	0	1	0	2	0	1
79	2	max	264.476	1	0	1	.048	10	0	1	0	10	0	1
80		min	-30.722	3	0	1	-13.366	4	0	1	-.001	4	0	1
81	3	max	264.541	1	0	1	.048	10	0	1	0	10	0	1
82		min	-30.674	3	0	1	-13.422	4	0	1	-.002	4	0	1
83	4	max	264.606	1	0	1	.048	10	0	1	0	10	0	1
84		min	-30.625	3	0	1	-13.478	4	0	1	-.004	4	0	1
85	5	max	264.67	1	0	1	.048	10	0	1	0	10	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-30.577	3	0	1	-13.534	4	0	1	-.005	4	0	1
87		6	max	264.735	1	0	1	.048	10	0	1	0	10	0	1
88			min	-30.528	3	0	1	-13.59	4	0	1	-.006	4	0	1
89		7	max	264.8	1	0	1	.048	10	0	1	0	10	0	1
90			min	-30.48	3	0	1	-13.646	4	0	1	-.007	4	0	1
91		8	max	264.865	1	0	1	.048	10	0	1	0	10	0	1
92			min	-30.431	3	0	1	-13.703	4	0	1	-.008	4	0	1
93		9	max	264.929	1	0	1	.048	10	0	1	0	10	0	1
94			min	-30.383	3	0	1	-13.759	4	0	1	-.01	4	0	1
95		10	max	264.994	1	0	1	.048	10	0	1	0	10	0	1
96			min	-30.334	3	0	1	-13.815	4	0	1	-.011	4	0	1
97		11	max	265.059	1	0	1	.048	10	0	1	0	10	0	1
98			min	-30.285	3	0	1	-13.871	4	0	1	-.012	4	0	1
99		12	max	265.123	1	0	1	.048	10	0	1	0	10	0	1
100			min	-30.237	3	0	1	-13.927	4	0	1	-.013	4	0	1
101		13	max	265.188	1	0	1	.048	10	0	1	0	10	0	1
102			min	-30.188	3	0	1	-13.983	4	0	1	-.015	4	0	1
103		14	max	265.253	1	0	1	.048	10	0	1	0	10	0	1
104			min	-30.14	3	0	1	-14.039	4	0	1	-.016	4	0	1
105		15	max	265.317	1	0	1	.048	10	0	1	0	10	0	1
106			min	-30.091	3	0	1	-14.095	4	0	1	-.017	4	0	1
107		16	max	265.382	1	0	1	.048	10	0	1	0	10	0	1
108			min	-30.043	3	0	1	-14.151	4	0	1	-.018	4	0	1
109		17	max	265.447	1	0	1	.048	10	0	1	0	10	0	1
110			min	-29.994	3	0	1	-14.207	4	0	1	-.02	4	0	1
111		18	max	265.512	1	0	1	.048	10	0	1	0	10	0	1
112			min	-29.946	3	0	1	-14.263	4	0	1	-.021	4	0	1
113		19	max	265.576	1	0	1	.048	10	0	1	0	10	0	1
114			min	-29.897	3	0	1	-14.319	4	0	1	-.022	4	0	1
115	M6	1	max	611.16	2	.628	6	.928	4	0	3	0	3	0	1
116			min	-1033.428	3	.141	15	-.296	3	0	5	0	1	0	1
117		2	max	611.277	2	.582	6	.822	4	0	3	0	3	0	15
118			min	-1033.34	3	.13	15	-.296	3	0	5	0	1	0	6
119		3	max	611.393	2	.536	6	.717	4	0	3	0	4	0	15
120			min	-1033.253	3	.119	15	-.296	3	0	5	0	1	0	6
121		4	max	611.509	2	.491	6	.611	4	0	3	0	4	0	15
122			min	-1033.166	3	.108	15	-.296	3	0	5	0	1	0	6
123		5	max	611.626	2	.445	6	.506	4	0	3	0	4	0	15
124			min	-1033.078	3	.098	15	-.296	3	0	5	0	1	0	6
125		6	max	611.742	2	.409	2	.4	4	0	3	0	4	0	15
126			min	-1032.991	3	.087	15	-.296	3	0	5	0	1	0	6
127		7	max	611.859	2	.373	2	.295	4	0	3	0	4	0	15
128			min	-1032.904	3	.076	15	-.296	3	0	5	0	3	0	6
129		8	max	611.975	2	.338	2	.19	4	0	3	0	4	0	15
130			min	-1032.817	3	.066	15	-.296	3	0	5	0	3	0	6
131		9	max	612.091	2	.302	2	.084	4	0	3	0	4	0	15
132			min	-1032.729	3	.053	12	-.296	3	0	5	0	3	0	6
133		10	max	612.208	2	.267	2	.013	9	0	3	0	4	0	15
134			min	-1032.642	3	.035	12	-.296	3	0	5	0	3	0	2
135		11	max	612.324	2	.231	2	.013	9	0	3	0	4	0	15
136			min	-1032.555	3	.017	3	-.296	3	0	5	0	3	0	2
137		12	max	612.441	2	.195	2	.013	9	0	3	0	4	0	15
138			min	-1032.467	3	-.01	3	-.296	3	0	5	0	3	0	2
139		13	max	612.557	2	.16	2	.013	9	0	3	0	4	0	15
140			min	-1032.38	3	-.037	3	-.343	5	0	5	0	3	0	2
141		14	max	612.673	2	.124	2	.013	9	0	3	0	4	0	15
142			min	-1032.293	3	-.063	3	-.449	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	612.79	2	.089	2	.013	9	0	3	0	4	0	15
144		min	-1032.205	3	-.09	3	-.554	5	0	5	0	3	0	2
145	16	max	612.906	2	.053	2	.013	9	0	3	0	4	0	15
146		min	-1032.118	3	-.117	3	-.66	5	0	5	0	3	0	2
147	17	max	613.023	2	.018	2	.013	9	0	3	0	4	0	12
148		min	-1032.031	3	-.143	3	-.765	5	0	5	0	3	0	2
149	18	max	613.139	2	-.018	2	.013	9	0	3	0	4	0	12
150		min	-1031.944	3	-.17	3	-.871	5	0	5	0	3	0	2
151	19	max	613.255	2	-.053	15	.013	9	0	3	0	9	0	12
152		min	-1031.856	3	-.197	3	-.976	5	0	5	0	3	0	2
153	M7	1	max	425.879	2	1.792	.026	3	0	9	0	4	0	2
154		min	-334.694	3	.427	15	-1.347	4	0	3	0	3	0	12
155	2	max	425.81	2	1.615	4	.026	3	0	9	0	4	0	2
156		min	-334.746	3	.385	15	-1.213	4	0	3	0	3	0	3
157	3	max	425.741	2	1.437	4	.026	3	0	9	0	9	0	2
158		min	-334.797	3	.343	15	-1.08	4	0	3	0	3	0	3
159	4	max	425.673	2	1.26	4	.026	3	0	9	0	9	0	2
160		min	-334.849	3	.302	15	-.946	4	0	3	0	3	0	3
161	5	max	425.604	2	1.083	4	.026	3	0	9	0	9	0	15
162		min	-334.9	3	.26	15	-.812	4	0	3	0	5	0	3
163	6	max	425.536	2	.906	4	.026	3	0	9	0	9	0	15
164		min	-334.952	3	.218	15	-.679	4	0	3	0	5	0	6
165	7	max	425.467	2	.729	4	.026	3	0	9	0	9	0	15
166		min	-335.003	3	.177	15	-.545	4	0	3	0	5	0	6
167	8	max	425.398	2	.551	4	.026	3	0	9	0	9	0	15
168		min	-335.055	3	.135	15	-.411	4	0	3	0	5	0	6
169	9	max	425.33	2	.374	4	.026	3	0	9	0	9	0	15
170		min	-335.106	3	.093	15	-.278	4	0	3	0	5	-.001	6
171	10	max	425.261	2	.211	2	.026	3	0	9	0	9	0	15
172		min	-335.158	3	.032	12	-.144	4	0	3	0	5	-.001	6
173	11	max	425.193	2	.072	2	.026	3	0	9	0	9	0	15
174		min	-335.209	3	-.063	3	-.011	4	0	3	0	5	-.001	6
175	12	max	425.124	2	-.032	15	.125	5	0	9	0	9	0	15
176		min	-335.26	3	-.167	3	-.003	9	0	3	0	5	-.001	6
177	13	max	425.055	2	-.073	15	.258	5	0	9	0	9	0	15
178		min	-335.312	3	-.335	6	-.003	9	0	3	0	5	-.001	6
179	14	max	424.987	2	-.115	15	.392	5	0	9	0	9	0	15
180		min	-335.363	3	-.512	6	-.003	9	0	3	0	5	-.001	6
181	15	max	424.918	2	-.157	15	.525	5	0	9	0	9	0	15
182		min	-335.415	3	-.689	6	-.003	9	0	3	0	5	0	6
183	16	max	424.85	2	-.198	15	.659	5	0	9	0	9	0	15
184		min	-335.466	3	-.867	6	-.003	9	0	3	0	5	0	6
185	17	max	424.781	2	-.24	15	.793	5	0	9	0	9	0	15
186		min	-335.518	3	-1.044	6	-.003	9	0	3	0	5	0	6
187	18	max	424.712	2	-.282	15	.926	5	0	9	0	9	0	15
188		min	-335.569	3	-1.221	6	-.003	9	0	3	0	3	0	6
189	19	max	424.644	2	-.323	15	1.06	5	0	9	0	9	0	1
190		min	-335.621	3	-1.398	6	-.003	9	0	3	0	3	0	1
191	M8	1	max	764.131	2	0	.076	9	0	1	0	4	0	1
192		min	-139.399	3	0	1	-13.594	4	0	1	0	3	0	1
193	2	max	764.196	2	0	1	.076	9	0	1	0	9	0	1
194		min	-139.35	3	0	1	-13.65	4	0	1	-.001	4	0	1
195	3	max	764.261	2	0	1	.076	9	0	1	0	9	0	1
196		min	-139.302	3	0	1	-13.707	4	0	1	-.002	4	0	1
197	4	max	764.325	2	0	1	.076	9	0	1	0	9	0	1
198		min	-139.253	3	0	1	-13.763	4	0	1	-.004	4	0	1
199	5	max	764.39	2	0	1	.076	9	0	1	0	9	0	1



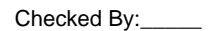
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-139.205	3	0	1	-13.819	4	0	1	-.005	4	0	1
201		6	max	764.455	2	0	1	.076	9	0	1	0	9	0	1
202			min	-139.156	3	0	1	-13.875	4	0	1	-.006	4	0	1
203		7	max	764.519	2	0	1	.076	9	0	1	0	9	0	1
204			min	-139.108	3	0	1	-13.931	4	0	1	-.007	4	0	1
205		8	max	764.584	2	0	1	.076	9	0	1	0	9	0	1
206			min	-139.059	3	0	1	-13.987	4	0	1	-.009	4	0	1
207		9	max	764.649	2	0	1	.076	9	0	1	0	9	0	1
208			min	-139.011	3	0	1	-14.043	4	0	1	-.01	4	0	1
209		10	max	764.714	2	0	1	.076	9	0	1	0	9	0	1
210			min	-138.962	3	0	1	-14.099	4	0	1	-.011	4	0	1
211		11	max	764.778	2	0	1	.076	9	0	1	0	9	0	1
212			min	-138.914	3	0	1	-14.155	4	0	1	-.012	4	0	1
213		12	max	764.843	2	0	1	.076	9	0	1	0	9	0	1
214			min	-138.865	3	0	1	-14.211	4	0	1	-.014	4	0	1
215		13	max	764.908	2	0	1	.076	9	0	1	0	9	0	1
216			min	-138.816	3	0	1	-14.267	4	0	1	-.015	4	0	1
217		14	max	764.972	2	0	1	.076	9	0	1	0	9	0	1
218			min	-138.768	3	0	1	-14.323	4	0	1	-.016	4	0	1
219		15	max	765.037	2	0	1	.076	9	0	1	0	9	0	1
220			min	-138.719	3	0	1	-14.379	4	0	1	-.018	4	0	1
221		16	max	765.102	2	0	1	.076	9	0	1	0	9	0	1
222			min	-138.671	3	0	1	-14.436	4	0	1	-.019	4	0	1
223		17	max	765.167	2	0	1	.076	9	0	1	0	9	0	1
224			min	-138.622	3	0	1	-14.492	4	0	1	-.02	4	0	1
225		18	max	765.231	2	0	1	.076	9	0	1	0	9	0	1
226			min	-138.574	3	0	1	-14.548	4	0	1	-.021	4	0	1
227		19	max	765.296	2	0	1	.076	9	0	1	0	9	0	1
228			min	-138.525	3	0	1	-14.604	4	0	1	-.023	4	0	1
229	M10	1	max	199.996	2	.673	4	1.028	5	0	1	0	9	0	1
230			min	-275.945	3	.171	15	-.069	1	-.001	5	0	3	0	1
231		2	max	200.113	2	.628	4	.923	5	0	1	0	4	0	15
232			min	-275.857	3	.16	15	-.069	1	-.001	5	0	3	0	4
233		3	max	200.229	2	.582	4	.817	5	0	1	0	4	0	15
234			min	-275.77	3	.149	15	-.069	1	-.001	5	0	3	0	4
235		4	max	200.346	2	.536	4	.712	5	0	1	0	4	0	15
236			min	-275.683	3	.139	15	-.069	1	-.001	5	0	3	0	4
237		5	max	200.462	2	.491	4	.606	5	0	1	0	4	0	15
238			min	-275.596	3	.128	15	-.069	1	-.001	5	0	3	0	4
239		6	max	200.578	2	.445	4	.501	5	0	1	0	4	0	15
240			min	-275.508	3	.117	15	-.069	1	-.001	5	0	3	0	4
241		7	max	200.695	2	.399	4	.395	5	0	1	0	5	0	15
242			min	-275.421	3	.107	15	-.069	1	-.001	5	0	3	0	4
243		8	max	200.811	2	.354	4	.29	5	0	1	0	5	0	15
244			min	-275.334	3	.096	15	-.069	1	-.001	5	0	3	0	4
245		9	max	200.928	2	.308	4	.184	5	0	1	0	5	0	15
246			min	-275.246	3	.085	15	-.069	1	-.001	5	0	3	0	4
247		10	max	201.044	2	.262	4	.079	5	0	1	0	5	0	15
248			min	-275.159	3	.074	15	-.069	1	-.001	5	0	3	0	4
249		11	max	201.16	2	.217	4	.005	10	0	1	0	5	0	15
250			min	-275.072	3	.061	12	-.069	1	-.001	5	0	3	0	4
251		12	max	201.277	2	.171	4	.005	10	0	1	0	5	0	15
252			min	-274.984	3	.044	12	-.145	4	-.001	5	0	3	0	4
253		13	max	201.393	2	.125	4	.005	10	0	1	0	5	0	15
254			min	-274.897	3	.026	12	-.25	4	-.001	5	0	3	0	4
255		14	max	201.51	2	.08	4	.005	10	0	1	0	5	0	15
256			min	-274.81	3	.008	12	-.356	4	-.001	5	0	3	0	4





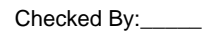
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-29.972	3	0	1	-12.742	5	0	1	-.005	5	0	1
315	6	max	265.038	1	0	1	.425	1	0	1	0	1	0	1
316		min	-29.923	3	0	1	-12.798	5	0	1	-.006	5	0	1
317	7	max	265.103	1	0	1	.425	1	0	1	0	1	0	1
318		min	-29.875	3	0	1	-12.854	5	0	1	-.007	5	0	1
319	8	max	265.168	1	0	1	.425	1	0	1	0	1	0	1
320		min	-29.826	3	0	1	-12.91	5	0	1	-.008	5	0	1
321	9	max	265.233	1	0	1	.425	1	0	1	0	1	0	1
322		min	-29.778	3	0	1	-12.967	5	0	1	-.009	5	0	1
323	10	max	265.297	1	0	1	.425	1	0	1	0	1	0	1
324		min	-29.729	3	0	1	-13.023	5	0	1	-.01	5	0	1
325	11	max	265.362	1	0	1	.425	1	0	1	0	1	0	1
326		min	-29.681	3	0	1	-13.079	5	0	1	-.011	5	0	1
327	12	max	265.427	1	0	1	.425	1	0	1	0	1	0	1
328		min	-29.632	3	0	1	-13.135	5	0	1	-.013	5	0	1
329	13	max	265.491	1	0	1	.425	1	0	1	0	1	0	1
330		min	-29.584	3	0	1	-13.191	5	0	1	-.014	5	0	1
331	14	max	265.556	1	0	1	.425	1	0	1	0	1	0	1
332		min	-29.535	3	0	1	-13.247	5	0	1	-.015	5	0	1
333	15	max	265.621	1	0	1	.425	1	0	1	0	1	0	1
334		min	-29.487	3	0	1	-13.303	5	0	1	-.016	5	0	1
335	16	max	265.685	1	0	1	.425	1	0	1	0	1	0	1
336		min	-29.438	3	0	1	-13.359	5	0	1	-.017	5	0	1
337	17	max	265.75	1	0	1	.425	1	0	1	0	1	0	1
338		min	-29.39	3	0	1	-13.415	5	0	1	-.019	5	0	1
339	18	max	265.815	1	0	1	.425	1	0	1	0	1	0	1
340		min	-29.341	3	0	1	-13.471	5	0	1	-.02	5	0	1
341	19	max	265.88	1	0	1	.425	1	0	1	0	1	0	1
342		min	-29.293	3	0	1	-13.527	5	0	1	-.021	5	0	1
343	M1	1	max	60.701	1	329.487	3	1.123	10	0	.025	4	0	2
344		min	3.26	10	-215.909	2	-14.518	4	0	3	-.002	10	0	3
345	2	max	60.819	1	329.297	3	1.123	10	0	2	.022	4	.047	2
346		min	3.358	10	-216.162	2	-14.276	4	0	3	-.002	10	-.072	3
347	3	max	60.171	3	4.53	4	1.119	10	0	5	.019	4	.093	2
348		min	-10.502	10	-19.391	2	-13.071	4	0	1	-.002	10	-.142	3
349	4	max	60.26	3	4.205	4	1.119	10	0	5	.016	4	.098	2
350		min	-10.404	10	-19.644	2	-12.829	4	0	1	-.001	10	-.138	3
351	5	max	60.348	3	3.881	4	1.119	10	0	5	.013	4	.102	2
352		min	-10.306	10	-19.897	2	-12.587	4	0	1	-.001	10	-.135	3
353	6	max	60.437	3	3.556	4	1.119	10	0	5	.01	4	.106	2
354		min	-10.207	10	-20.15	2	-12.345	4	0	1	0	10	-.131	3
355	7	max	60.525	3	3.294	14	1.119	10	0	5	.008	4	.111	2
356		min	-10.109	10	-20.403	2	-12.103	4	0	1	0	10	-.127	3
357	8	max	60.614	3	3.045	14	1.119	10	0	5	.005	4	.115	2
358		min	-10.011	10	-20.656	2	-11.861	4	0	1	0	10	-.124	3
359	9	max	60.702	3	2.797	14	1.119	10	0	5	.003	3	.119	2
360		min	-9.912	10	-20.909	2	-11.619	4	0	1	0	10	-.12	3
361	10	max	60.791	3	2.548	14	1.119	10	0	5	.002	3	.124	2
362		min	-9.814	10	-21.162	2	-11.445	1	0	1	0	10	-.116	3
363	11	max	60.879	3	2.299	14	1.119	10	0	5	0	3	.129	2
364		min	-9.716	10	-21.415	2	-11.445	1	0	1	-.002	1	-.112	3
365	12	max	60.968	3	2.051	14	1.119	10	0	5	0	10	.133	2
366		min	-9.617	10	-21.669	2	-11.445	1	0	1	-.005	1	-.108	3
367	13	max	61.056	3	1.802	14	1.119	10	0	5	0	10	.138	2
368		min	-9.519	10	-21.922	2	-11.445	1	0	1	-.007	1	-.104	3
369	14	max	61.145	3	1.553	14	1.119	10	0	5	0	10	.143	2
370		min	-9.421	10	-22.175	2	-11.445	1	0	1	-.01	1	-.1	3





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-10.001	10	-19.875	2	-18.914	5	0	10	-.012	1	-.134	3
429	6	max	59.79	3	3.184	9	11.445	1	0	1	.029	5	.106	2
430		min	-9.903	10	-20.128	2	-18.672	5	0	10	-.01	1	-.131	3
431	7	max	59.879	3	2.973	9	11.445	1	0	1	.024	5	.111	2
432		min	-9.805	10	-20.381	2	-18.43	5	0	10	-.008	1	-.127	3
433	8	max	59.967	3	2.762	9	11.445	1	0	1	.021	5	.115	2
434		min	-9.706	10	-20.634	2	-18.188	5	0	10	-.005	1	-.124	3
435	9	max	60.056	3	2.551	9	11.445	1	0	1	.017	5	.119	2
436		min	-9.608	10	-20.887	2	-17.946	5	0	10	-.003	1	-.12	3
437	10	max	60.144	3	2.34	9	11.445	1	0	1	.013	4	.124	2
438		min	-9.51	10	-21.14	2	-17.704	5	0	10	0	1	-.116	3
439	11	max	60.233	3	2.129	9	11.445	1	0	1	.01	3	.129	2
440		min	-9.411	10	-21.393	2	-17.462	5	0	10	0	10	-.112	3
441	12	max	60.321	3	1.918	9	11.445	1	0	1	.009	3	.133	2
442		min	-9.313	10	-21.646	2	-17.22	5	0	10	0	10	-.108	3
443	13	max	60.41	3	1.707	9	11.445	1	0	1	.009	3	.138	2
444		min	-9.215	10	-21.899	2	-16.978	5	0	10	0	10	-.104	3
445	14	max	60.498	3	1.496	9	11.445	1	0	1	.01	1	.143	2
446		min	-9.116	10	-22.152	2	-16.736	5	0	10	-.002	5	-.1	3
447	15	max	60.587	3	1.285	9	11.445	1	0	1	.012	1	.148	2
448		min	-9.018	10	-22.405	2	-16.494	5	0	10	-.006	5	-.096	3
449	16	max	83.177	2	86.711	2	11.539	1	0	10	.015	1	.152	2
450		min	-20.678	3	-121.947	3	-15.116	5	0	4	-.009	5	-.092	3
451	17	max	83.295	2	86.458	2	11.539	1	0	10	.017	1	.133	2
452		min	-20.59	3	-122.136	3	-14.874	5	0	4	-.012	5	-.065	3
453	18	max	8.599	5	307.312	2	11.967	1	0	2	.02	1	.067	2
454		min	-60.797	1	-149.701	3	-27.149	5	0	3	-.018	5	-.033	3
455	19	max	8.654	5	307.059	2	11.967	1	0	2	.023	1	0	2
456		min	-60.679	1	-149.89	3	-26.907	5	0	3	-.024	5	0	3
457	M13	1	max	100.844	4	215.849	2	-.282	15	0	.023	1	0	2
458		min	-1.123	10	-329.452	3	-60.698	1	0	3	-.002	10	0	3
459	2	max	97.025	4	154.785	2	.314	5	0	2	.015	3	.11	3
460		min	-1.123	10	-235.305	3	-45.228	1	0	3	-.004	2	-.072	2
461	3	max	93.205	4	93.722	2	1.183	5	0	2	.012	3	.183	3
462		min	-1.123	10	-141.158	3	-29.758	1	0	3	-.013	1	-.12	2
463	4	max	89.386	4	32.658	2	2.052	5	0	2	.009	3	.22	3
464		min	-1.123	10	-47.012	3	-14.287	1	0	3	-.021	1	-.145	2
465	5	max	85.566	4	47.135	3	4.603	2	0	2	.006	3	.22	3
466		min	-1.123	10	-28.405	2	-6.945	3	0	3	-.024	1	-.146	2
467	6	max	81.746	4	141.281	3	16.653	1	0	2	.003	5	.183	3
468		min	-1.123	10	-89.469	2	-6.127	3	0	3	-.02	1	-.123	2
469	7	max	80.188	3	235.428	3	32.123	1	0	2	.005	5	.11	3
470		min	-1.123	10	-150.532	2	-5.31	3	0	3	-.011	1	-.076	2
471	8	max	80.188	3	329.575	3	47.594	1	0	2	.007	4	0	5
472		min	-1.123	10	-211.596	2	-4.492	3	0	3	0	3	-.006	2
473	9	max	80.188	3	423.721	3	63.064	1	0	2	.026	1	.088	2
474		min	-1.123	10	-272.659	2	-3.674	3	0	3	-.002	3	-.147	3
475	10	max	80.188	3	-6.687	15	78.534	1	0	2	.054	1	.206	2
476		min	-1.123	10	-517.868	3	2.017	12	0	3	-.015	3	-.33	3
477	11	max	45.059	4	272.659	2	6.822	5	0	3	.026	1	.088	2
478		min	-1.123	10	-423.721	3	-63.064	1	0	2	-.014	3	-.147	3
479	12	max	41.239	4	211.596	2	7.691	5	0	3	.007	2	0	5
480		min	-1.123	10	-329.574	3	-47.593	1	0	2	-.012	3	-.006	2
481	13	max	37.42	4	150.532	2	8.56	5	0	3	.001	10	.11	3
482		min	-1.123	10	-235.428	3	-32.123	1	0	2	-.011	1	-.076	2
483	14	max	33.6	4	89.469	2	9.429	5	0	3	-.001	10	.183	3
484		min	-1.123	10	-141.281	3	-16.653	1	0	2	-.02	1	-.123	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	29.781	4	28.405	2	10.66	4	0	3	0	5	.22	3
486			min	-1.123	10	-47.135	3	-4.603	2	0	2	-.024	1	-.146	2
487		16	max	25.961	4	47.012	3	14.752	4	0	3	.005	5	.22	3
488			min	-1.123	10	-32.658	2	-1.396	10	0	2	-.021	1	-.145	2
489		17	max	22.141	4	141.159	3	29.758	1	0	3	.01	5	.183	3
490			min	-1.123	10	-93.722	2	.156	10	0	2	-.013	1	-.12	2
491		18	max	18.322	4	235.305	3	45.228	1	0	3	.016	4	.11	3
492			min	-1.123	10	-154.785	2	1.708	10	0	2	-.004	2	-.072	2
493		19	max	14.502	4	329.452	3	60.698	1	0	3	.025	4	0	2
494			min	-1.123	10	-215.849	2	3.26	10	0	2	-.002	10	0	3
495	M16	1	max	26.897	5	307.133	2	8.654	5	0	3	.023	1	0	2
496			min	-11.951	1	-149.914	3	-60.682	1	0	2	-.024	5	0	3
497		2	max	23.078	5	219.915	2	9.523	5	0	3	.003	9	.05	3
498			min	-11.951	1	-108.077	3	-45.212	1	0	2	-.02	5	-.102	2
499		3	max	19.258	5	132.697	2	10.393	5	0	3	0	3	.084	3
500			min	-11.951	1	-66.239	3	-29.742	1	0	2	-.018	4	-.171	2
501		4	max	15.439	5	45.479	2	11.262	5	0	3	-.001	12	.102	3
502			min	-11.951	1	-24.401	3	-14.272	1	0	2	-.021	1	-.206	2
503		5	max	11.619	5	17.436	3	12.131	5	0	3	-.002	12	.103	3
504			min	-11.951	1	-41.739	2	-4.405	3	0	2	-.024	1	-.206	2
505		6	max	7.8	5	59.274	3	16.669	1	0	3	-.001	10	.088	3
506			min	-11.951	1	-128.956	2	-3.587	3	0	2	-.02	1	-.173	2
507		7	max	3.98	5	101.111	3	32.139	1	0	3	.003	5	.057	3
508			min	-11.951	1	-216.174	2	-2.769	3	0	2	-.011	1	-.106	2
509		8	max	2.489	3	142.949	3	47.61	1	0	3	.009	4	.009	3
510			min	-11.951	1	-303.392	2	-1.952	3	0	2	-.008	3	-.005	2
511		9	max	2.489	3	184.786	3	63.08	1	0	3	.026	1	.13	2
512			min	-11.951	1	-390.61	2	-1.134	3	0	2	-.008	3	-.054	3
513		10	max	16.054	5	-6.575	15	78.55	1	0	14	.054	1	.299	2
514			min	-11.951	1	-477.828	2	-1.105	3	0	2	-.008	3	-.134	3
515		11	max	12.235	5	390.61	2	5.587	5	0	2	.026	1	.13	2
516			min	-11.951	1	-184.786	3	-63.08	1	0	3	-.009	5	-.054	3
517		12	max	8.415	5	303.392	2	6.456	5	0	2	.007	2	.009	3
518			min	-11.951	1	-142.949	3	-47.609	1	0	3	-.007	5	-.005	2
519		13	max	4.596	5	216.174	2	7.325	5	0	2	.001	10	.057	3
520			min	-11.951	1	-101.111	3	-32.139	1	0	3	-.011	1	-.106	2
521		14	max	1.176	10	128.956	2	8.195	5	0	2	0	15	.088	3
522			min	-11.951	1	-59.274	3	-16.669	1	0	3	-.02	1	-.173	2
523		15	max	1.176	10	41.739	2	9.403	4	0	2	.002	5	.103	3
524			min	-11.951	1	-17.436	3	-4.607	2	0	3	-.024	1	-.206	2
525		16	max	1.176	10	24.402	3	14.272	1	0	2	.006	5	.102	3
526			min	-11.951	1	-45.479	2	-1.396	10	0	3	-.021	1	-.206	2
527		17	max	1.176	10	66.239	3	29.742	1	0	2	.01	5	.084	3
528			min	-13.647	4	-132.697	2	.156	10	0	3	-.013	1	-.171	2
529		18	max	1.176	10	108.077	3	45.212	1	0	2	.016	4	.05	3
530			min	-17.466	4	-219.915	2	1.708	10	0	3	-.004	2	-.102	2
531		19	max	1.176	10	149.914	3	60.683	1	0	2	.025	4	0	2
532			min	-21.286	4	-307.133	2	3.26	10	0	3	-.002	10	0	5
533	M15	1	max	0	1	.731	3	.163	3	0	1	0	1	0	1
534			min	-110.834	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.65	3	.163	3	0	1	0	1	0	1
536			min	-110.899	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.569	3	.163	3	0	1	0	1	0	1
538			min	-110.964	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.487	3	.163	3	0	1	0	1	0	1
540			min	-111.029	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.406	3	.163	3	0	1	0	1	0	1



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-111.094	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.325	3	.163	3	0	1	0	1	0	1
544		min	-111.16	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.244	3	.163	3	0	1	0	3	0	1
546		min	-111.225	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.162	3	.163	3	0	1	0	3	0	1
548		min	-111.29	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.081	3	.163	3	0	1	0	3	0	1
550		min	-111.355	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.163	3	0	1	0	3	0	1
552		min	-111.42	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.163	3	0	1	0	3	0	1
554		min	-111.485	3	-.081	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.163	3	0	1	0	3	0	1
556		min	-111.551	3	-.162	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.163	3	0	1	0	3	0	1
558		min	-111.616	3	-.244	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.163	3	0	1	0	3	0	1
560		min	-111.681	3	-.325	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.163	3	0	1	0	3	0	1
562		min	-111.746	3	-.406	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.163	3	0	1	0	3	0	1
564		min	-111.811	3	-.487	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.163	3	0	1	0	3	0	1
566		min	-111.877	3	-.569	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.163	3	0	1	0	3	0	1
568		min	-111.942	3	-.65	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.163	3	0	1	0	3	0	1
570		min	-112.007	3	-.731	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.908	.298	4	0	3	0	3	0	1
572		min	-159.044	4	0	1	-.067	3	0	4	0	4	0	1
573	2	max	0	1	1.696	4	.269	4	0	3	0	3	0	1
574		min	-159.023	4	0	1	-.067	3	0	4	0	4	0	4
575	3	max	0	1	1.484	4	.239	4	0	3	0	3	0	1
576		min	-159.003	4	0	1	-.067	3	0	4	0	4	0	4
577	4	max	0	1	1.272	4	.21	4	0	3	0	3	0	1
578		min	-158.982	4	0	1	-.067	3	0	4	0	4	-.001	4
579	5	max	0	1	1.06	4	.18	4	0	3	0	3	0	1
580		min	-158.962	4	0	1	-.067	3	0	4	0	9	-.002	4
581	6	max	0	1	.848	4	.151	4	0	3	0	3	0	1
582		min	-158.941	4	0	1	-.067	3	0	4	0	9	-.002	4
583	7	max	0	1	.636	4	.121	4	0	3	0	3	0	1
584		min	-158.92	4	0	1	-.067	3	0	4	0	9	-.002	4
585	8	max	0	1	.424	4	.092	4	0	3	0	5	0	1
586		min	-158.9	4	0	1	-.067	3	0	4	0	9	-.002	4
587	9	max	0	1	.212	4	.062	4	0	3	0	5	0	1
588		min	-158.879	4	0	1	-.067	3	0	4	0	9	-.002	4
589	10	max	0	1	0	1	.033	4	0	3	0	5	0	1
590		min	-158.859	4	0	1	-.067	3	0	4	0	9	-.002	4
591	11	max	0	1	0	1	.015	9	0	3	0	5	0	1
592		min	-158.838	4	-.212	4	-.067	3	0	4	0	9	-.002	4
593	12	max	0	1	0	1	.015	9	0	3	0	5	0	1
594		min	-158.818	4	-.424	4	-.067	3	0	4	0	9	-.002	4
595	13	max	0	1	0	1	.015	9	0	3	0	5	0	1
596		min	-158.797	4	-.636	4	-.067	3	0	4	0	3	-.002	4
597	14	max	0	1	0	1	.015	9	0	3	0	5	0	1
598		min	-158.777	4	-.848	4	-.089	5	0	4	0	3	-.002	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.019	9	0	1	.015	9	0	3	0	5	0	1
600		min	-158.756	4	-1.06	4	-.118	5	0	4	0	3	-.002	4
601	16	max	.092	9	0	1	.015	9	0	3	0	5	0	1
602		min	-158.794	5	-1.272	4	-.148	5	0	4	0	3	-.001	4
603	17	max	.164	9	0	1	.015	9	0	3	0	9	0	1
604		min	-158.844	5	-1.484	4	-.177	5	0	4	0	3	0	4
605	18	max	.237	9	0	1	.015	9	0	3	0	9	0	1
606		min	-158.894	5	-1.696	4	-.207	5	0	4	0	3	0	4
607	19	max	.309	9	0	1	.015	9	0	3	0	9	0	1
608		min	-158.945	5	-1.908	4	-.236	5	0	4	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.008	2	.001	9	7.583e-4	5	NC	3	NC	1
2			min	-.003	3	-.007	3	-.008	5	-1.888e-4	3	4683.096	2	NC	1
3		2	max	.002	2	.007	2	.001	9	7.783e-4	5	NC	3	NC	1
4			min	-.003	3	-.007	3	-.008	5	-1.788e-4	3	5106.954	2	NC	1
5		3	max	.002	2	.006	2	.001	9	7.983e-4	5	NC	1	NC	1
6			min	-.003	3	-.007	3	-.008	5	-1.689e-4	3	5610.453	2	NC	1
7		4	max	.001	2	.006	2	.001	9	8.182e-4	5	NC	1	NC	1
8			min	-.003	3	-.007	3	-.008	5	-1.59e-4	3	6212.875	2	NC	1
9		5	max	.001	2	.005	2	.001	9	8.382e-4	5	NC	1	NC	1
10			min	-.002	3	-.006	3	-.007	5	-1.49e-4	3	6939.946	2	NC	1
11		6	max	.001	2	.005	2	0	9	8.581e-4	5	NC	1	NC	1
12			min	-.002	3	-.006	3	-.007	5	-1.391e-4	3	7826.57	2	NC	1
13		7	max	.001	2	.004	2	0	9	8.781e-4	5	NC	1	NC	1
14			min	-.002	3	-.006	3	-.007	5	-1.305e-4	1	8921.027	2	NC	1
15		8	max	.001	2	.004	2	0	9	8.981e-4	5	NC	1	NC	1
16			min	-.002	3	-.005	3	-.006	5	-1.221e-4	1	NC	1	NC	1
17		9	max	0	2	.003	2	0	9	9.18e-4	5	NC	1	NC	1
18			min	-.002	3	-.005	3	-.006	5	-1.136e-4	1	NC	1	NC	1
19		10	max	0	2	.003	2	0	9	9.38e-4	5	NC	1	NC	1
20		min	-.002	3	-.005	3	-.006	5	-1.052e-4	1	NC	1	NC	1	
21	11	max	0	2	.002	2	0	9	9.58e-4	5	NC	1	NC	1	
22		min	-.001	3	-.004	3	-.005	5	-9.678e-5	1	NC	1	NC	1	
23	12	max	0	2	.002	2	0	9	9.779e-4	5	NC	1	NC	1	
24		min	-.001	3	-.004	3	-.005	5	-8.835e-5	1	NC	1	NC	1	
25	13	max	0	2	.001	2	0	9	9.979e-4	5	NC	1	NC	1	
26		min	-.001	3	-.003	3	-.004	5	-7.991e-5	1	NC	1	NC	1	
27	14	max	0	2	0	2	0	9	1.018e-3	5	NC	1	NC	1	
28		min	0	3	-.003	3	-.003	5	-7.148e-5	1	NC	1	NC	1	
29	15	max	0	2	0	2	0	9	1.038e-3	5	NC	1	NC	1	
30		min	0	3	-.002	3	-.003	5	-6.304e-5	1	NC	1	NC	1	
31	16	max	0	2	0	2	0	9	1.058e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.002	5	-5.461e-5	1	NC	1	NC	1	
33	17	max	0	2	0	2	0	9	1.078e-3	5	NC	1	NC	1	
34		min	0	3	-.001	3	-.001	5	-4.617e-5	1	NC	1	NC	1	
35	18	max	0	2	0	2	0	9	1.098e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-3.774e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.118e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-2.93e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.372e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.198e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	1.991e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	9	-5.219e-4	5	NC	1	NC	1



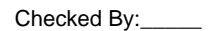
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.005	5	2.61e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.24e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.008	5	3.229e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	9	-5.261e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.011	4	3.847e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.282e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.013	4	4.466e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.304e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.016	4	5.085e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-5.325e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.019	4	5.704e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.346e-4	5	NC	1	NC	1
55		9	max	0	3	0	2	.021	4	6.323e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-5.367e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.023	4	6.942e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.388e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.026	4	7.561e-5	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-5.41e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.028	4	8.18e-5	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-5.431e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.03	4	8.799e-5	1	NC	1	NC	1
64			min	-.001	2	-.007	3	0	10	-5.452e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.032	4	9.418e-5	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	10	-5.473e-4	5	NC	1	NC	1
67		15	max	.001	3	.004	2	.034	4	1.004e-4	1	NC	1	NC	1
68			min	-.001	2	-.007	3	0	10	-5.494e-4	5	NC	1	NC	1
69		16	max	.001	3	.005	2	.036	4	1.066e-4	1	NC	1	NC	1
70			min	-.001	2	-.007	3	0	10	-5.516e-4	5	8625.434	2	NC	1
71		17	max	.001	3	.006	2	.037	4	1.127e-4	1	NC	1	NC	1
72			min	-.001	2	-.008	3	0	10	-5.537e-4	5	7356.474	2	NC	1
73		18	max	.001	3	.007	2	.039	4	1.189e-4	1	NC	1	NC	1
74			min	-.002	2	-.007	3	0	10	-5.558e-4	5	6377.183	2	NC	1
75		19	max	.001	3	.008	2	.041	4	1.251e-4	1	NC	3	NC	1
76			min	-.002	2	-.007	3	0	10	-5.579e-4	5	5613.214	2	NC	1
77	M4	1	max	.001	1	.009	2	0	10	2.392e-3	5	NC	1	NC	1
78			min	0	3	-.008	3	-.043	4	-1.345e-4	1	NC	1	447.418	4
79		2	max	.001	1	.008	2	0	10	2.392e-3	5	NC	1	NC	1
80			min	0	3	-.007	3	-.04	4	-1.345e-4	1	NC	1	487.671	4
81		3	max	.001	1	.008	2	0	10	2.392e-3	5	NC	1	NC	1
82			min	0	3	-.007	3	-.036	4	-1.345e-4	1	NC	1	535.571	4
83		4	max	.001	1	.007	2	0	10	2.392e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.033	4	-1.345e-4	1	NC	1	593.131	4
85		5	max	0	1	.007	2	0	10	2.392e-3	5	NC	1	NC	1
86			min	0	3	-.006	3	-.029	4	-1.345e-4	1	NC	1	663.094	4
87		6	max	0	1	.006	2	0	10	2.392e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.026	4	-1.345e-4	1	NC	1	749.275	4
89		7	max	0	1	.006	2	0	10	2.392e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.023	4	-1.345e-4	1	NC	1	857.105	4
91		8	max	0	1	.005	2	0	10	2.392e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.019	4	-1.345e-4	1	NC	1	994.536	4
93		9	max	0	1	.005	2	0	10	2.392e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.016	4	-1.345e-4	1	NC	1	1173.598	4
95		10	max	0	1	.004	2	0	10	2.392e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.014	4	-1.345e-4	1	NC	1	1413.195	4
97		11	max	0	1	.004	2	0	10	2.392e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.011	4	-1.345e-4	1	NC	1	1744.462	4
99		12	max	0	1	.003	2	0	10	2.392e-3	5	NC	1	NC	1





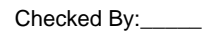
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	4.454e-6	9	NC	1	NC	1
158			min	0	2	-.003	3	0	9	-5.271e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.008	4	3.843e-6	9	NC	1	NC	1
160			min	0	2	-.005	3	0	9	-5.197e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.011	4	3.233e-6	9	NC	1	NC	1
162			min	-.001	2	-.007	3	0	9	-5.123e-4	4	NC	1	NC	1
163		6	max	.001	3	.005	2	.014	4	1.819e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	9	-5.05e-4	4	8505.821	2	NC	1
165		7	max	.001	3	.007	2	.017	4	4.153e-5	3	NC	1	NC	1
166			min	-.002	2	-.01	3	0	9	-4.976e-4	4	7053.472	2	NC	1
167		8	max	.001	3	.008	2	.019	4	6.486e-5	3	NC	1	NC	1
168			min	-.002	2	-.011	3	0	9	-4.902e-4	4	5983.524	2	NC	1
169		9	max	.002	3	.009	2	.022	4	8.819e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	9	-4.828e-4	4	5157.774	2	NC	1
171		10	max	.002	3	.01	2	.024	4	1.115e-4	3	NC	3	NC	1
172			min	-.002	2	-.014	3	0	9	-4.755e-4	4	4499.64	2	NC	1
173		11	max	.002	3	.012	2	.027	4	1.349e-4	3	NC	3	NC	1
174			min	-.003	2	-.015	3	0	9	-4.681e-4	4	3963.054	2	NC	1
175		12	max	.002	3	.013	2	.029	4	1.582e-4	3	NC	3	NC	1
176			min	-.003	2	-.016	3	0	9	-4.607e-4	4	3518.394	2	NC	1
177		13	max	.003	3	.015	2	.031	4	1.815e-4	3	NC	3	NC	1
178			min	-.003	2	-.017	3	0	9	-4.533e-4	4	3145.531	2	NC	1
179		14	max	.003	3	.016	2	.033	4	2.049e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	0	9	-4.459e-4	4	2830.137	2	NC	1
181		15	max	.003	3	.018	2	.035	4	2.282e-4	3	NC	3	NC	1
182			min	-.004	2	-.019	3	0	9	-4.386e-4	4	2561.624	2	NC	1
183		16	max	.003	3	.02	2	.037	4	2.515e-4	3	NC	3	NC	1
184			min	-.004	2	-.02	3	0	9	-4.312e-4	4	2331.919	2	NC	1
185		17	max	.003	3	.022	2	.039	4	2.749e-4	3	NC	3	NC	1
186			min	-.004	2	-.02	3	0	9	-4.238e-4	4	2134.73	2	NC	1
187		18	max	.004	3	.023	2	.04	4	2.982e-4	3	NC	3	NC	1
188			min	-.005	2	-.021	3	0	9	-4.164e-4	4	1965.059	2	NC	1
189		19	max	.004	3	.025	2	.042	4	3.215e-4	3	NC	3	NC	1
190			min	-.005	2	-.022	3	0	9	-4.09e-4	4	1818.892	2	NC	1
191	M8	1	max	.004	2	.027	2	0	9	2.253e-3	4	NC	1	NC	1
192			min	0	3	-.022	3	-.044	4	-2.366e-4	3	NC	1	438.328	4
193		2	max	.003	2	.025	2	0	9	2.253e-3	4	NC	1	NC	1
194			min	0	3	-.02	3	-.04	4	-2.366e-4	3	NC	1	477.766	4
195		3	max	.003	2	.024	2	0	9	2.253e-3	4	NC	1	NC	1
196			min	0	3	-.019	3	-.037	4	-2.366e-4	3	NC	1	524.696	4
197		4	max	.003	2	.022	2	0	9	2.253e-3	4	NC	1	NC	1
198			min	0	3	-.018	3	-.033	4	-2.366e-4	3	NC	1	581.091	4
199		5	max	.003	2	.021	2	0	9	2.253e-3	4	NC	1	NC	1
200			min	0	3	-.017	3	-.03	4	-2.366e-4	3	NC	1	649.639	4
201		6	max	.003	2	.019	2	0	9	2.253e-3	4	NC	1	NC	1
202			min	0	3	-.016	3	-.026	4	-2.366e-4	3	NC	1	734.076	4
203		7	max	.002	2	.018	2	0	9	2.253e-3	4	NC	1	NC	1
204			min	0	3	-.014	3	-.023	4	-2.366e-4	3	NC	1	839.726	4
205		8	max	.002	2	.016	2	0	9	2.253e-3	4	NC	1	NC	1
206			min	0	3	-.013	3	-.02	4	-2.366e-4	3	NC	1	974.379	4
207		9	max	.002	2	.015	2	0	9	2.253e-3	4	NC	1	NC	1
208			min	0	3	-.012	3	-.017	4	-2.366e-4	3	NC	1	1149.822	4
209		10	max	.002	2	.013	2	0	9	2.253e-3	4	NC	1	NC	1
210			min	0	3	-.011	3	-.014	4	-2.366e-4	3	NC	1	1384.579	4
211		11	max	.002	2	.012	2	0	9	2.253e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.011	4	-2.366e-4	3	NC	1	1709.156	4
213		12	max	.001	2	.01	2	0	9	2.253e-3	4	NC	1	NC	1



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 25° 160mph 30psf 3.5ft 7-10.rdb Page 36



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.643e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-4.991e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.007	4	3.36e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.332e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.009	4	1.077e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.672e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.011	4	4.352e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-6.012e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.014	4	4.965e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-6.353e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.016	5	5.577e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-6.693e-4	4	NC	1	NC	1
283		9	max	0	3	0	2	.018	5	6.19e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-7.033e-4	4	NC	1	NC	1
285		10	max	0	3	.001	2	.02	5	6.803e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-7.374e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.022	5	7.415e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-7.714e-4	4	NC	1	NC	1
289		12	max	0	3	.002	2	.024	5	8.028e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-8.054e-4	4	NC	1	NC	1
291		13	max	0	3	.003	2	.026	5	8.64e-6	10	NC	1	NC	1
292			min	-.001	2	-.007	3	-.003	3	-8.394e-4	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.028	5	9.253e-6	10	NC	1	NC	1
294			min	-.001	2	-.007	3	-.003	3	-8.735e-4	4	NC	1	NC	1
295		15	max	.001	3	.004	2	.03	5	9.865e-6	10	NC	1	NC	1
296			min	-.001	2	-.007	3	-.003	3	-9.075e-4	4	NC	1	NC	1
297		16	max	.001	3	.005	2	.032	5	1.048e-5	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.003	3	-9.415e-4	4	8635.873	2	NC	1
299		17	max	.001	3	.006	2	.034	5	1.109e-5	10	NC	1	NC	1
300			min	-.001	2	-.008	3	-.003	3	-9.756e-4	4	7364.503	2	NC	1
301		18	max	.001	3	.007	2	.035	5	1.17e-5	10	NC	1	NC	1
302			min	-.002	2	-.008	3	-.002	3	-1.01e-3	4	6383.544	2	NC	1
303		19	max	.001	3	.008	2	.037	5	1.232e-5	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.002	3	-1.044e-3	4	5618.394	2	NC	1
305	M12	1	max	.001	1	.009	2	.001	1	2.8e-3	4	NC	1	NC	1
306			min	0	3	-.008	3	-.041	5	-1.352e-5	10	NC	1	474.996	5
307		2	max	.001	1	.008	2	.001	1	2.8e-3	4	NC	1	NC	1
308			min	0	3	-.007	3	-.037	5	-1.352e-5	10	NC	1	517.718	5
309		3	max	.001	1	.008	2	.001	1	2.8e-3	4	NC	1	NC	1
310			min	0	3	-.007	3	-.034	5	-1.352e-5	10	NC	1	568.555	5
311		4	max	.001	1	.007	2	.001	1	2.8e-3	4	NC	1	NC	1
312			min	0	3	-.006	3	-.031	5	-1.352e-5	10	NC	1	629.643	5
313		5	max	0	1	.007	2	0	1	2.8e-3	4	NC	1	NC	1
314			min	0	3	-.006	3	-.027	5	-1.352e-5	10	NC	1	703.893	5
315		6	max	0	1	.006	2	0	1	2.8e-3	4	NC	1	NC	1
316			min	0	3	-.005	3	-.024	5	-1.352e-5	10	NC	1	795.352	5
317		7	max	0	1	.006	2	0	1	2.8e-3	4	NC	1	NC	1
318			min	0	3	-.005	3	-.021	5	-1.352e-5	10	NC	1	909.784	5
319		8	max	0	1	.005	2	0	1	2.8e-3	4	NC	1	NC	1
320			min	0	3	-.005	3	-.018	5	-1.352e-5	10	NC	1	1055.628	5
321		9	max	0	1	.005	2	0	1	2.8e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.016	5	-1.352e-5	10	NC	1	1245.645	5
323		10	max	0	1	.004	2	0	1	2.8e-3	4	NC	1	NC	1
324			min	0	3	-.004	3	-.013	5	-1.352e-5	10	NC	1	1499.897	5
325		11	max	0	1	.004	2	0	1	2.8e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.01	5	-1.352e-5	10	NC	1	1851.42	5
327		12	max	0	1	.003	2	0	1	2.8e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.003	3	-.008	5	-1.352e-5	10	NC	1	2357.834	5
329		13	max	0	1	.003	2	0	1	2.8e-3	4	NC	1	NC	1
330			min	0	3	-.003	3	-.006	5	-1.352e-5	10	NC	1	3127.208	5
331		14	max	0	1	.002	2	0	1	2.8e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.004	5	-1.352e-5	10	NC	1	4382.436	5
333		15	max	0	1	.002	2	0	1	2.8e-3	4	NC	1	NC	1
334			min	0	3	-.002	3	-.003	5	-1.352e-5	10	NC	1	6647.126	5
335		16	max	0	1	.001	2	0	1	2.8e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.002	5	-1.352e-5	10	NC	1	NC	1
337		17	max	0	1	0	2	0	1	2.8e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-1.352e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.8e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-1.352e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.8e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.352e-5	10	NC	1	NC	1
343	M1	1	max	.007	3	.022	3	.005	5	4.721e-3	2	NC	1	NC	1
344			min	-.007	2	-.019	2	0	9	-6.787e-3	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.007	5	2.335e-3	2	NC	4	NC	1
346			min	-.007	2	-.01	2	-.001	9	-3.331e-3	3	5047.71	3	NC	1
347		3	max	.007	3	.004	3	.008	5	2.186e-4	5	NC	4	NC	1
348			min	-.007	2	-.003	2	-.002	9	-7.311e-5	1	2617.759	3	NC	1
349		4	max	.007	3	.004	2	.01	5	2.15e-4	5	NC	4	NC	1
350			min	-.007	2	-.003	3	-.002	9	-6.044e-5	9	1869.633	3	8716.069	5
351		5	max	.007	3	.01	2	.013	5	2.114e-4	5	NC	4	NC	1
352			min	-.007	2	-.01	3	-.002	1	-4.838e-5	9	1513.75	3	6171.568	5
353		6	max	.007	3	.015	2	.015	5	2.077e-4	5	NC	4	NC	1
354			min	-.007	2	-.015	3	-.002	1	-3.632e-5	9	1315.927	3	4702.785	5
355		7	max	.007	3	.019	2	.017	5	2.041e-4	5	NC	4	NC	1
356			min	-.007	2	-.018	3	-.001	9	-2.427e-5	9	1195.891	2	3761.363	5
357		8	max	.007	3	.022	2	.02	5	2.005e-4	5	NC	4	NC	1
358			min	-.007	2	-.021	3	-.001	9	-1.221e-5	9	1104.483	2	3114.975	5
359		9	max	.007	3	.024	2	.023	5	1.986e-4	4	NC	4	NC	1
360			min	-.007	2	-.022	3	0	9	-1.124e-6	10	1050.36	2	2649.028	5
361		10	max	.007	3	.024	2	.025	4	1.989e-4	4	NC	4	NC	1
362			min	-.007	2	-.023	3	0	9	-2.623e-6	10	1025.355	2	2287.313	4
363		11	max	.007	3	.024	2	.028	4	1.992e-4	4	NC	4	NC	1
364			min	-.007	2	-.022	3	0	10	-4.122e-6	10	1026.524	2	2010.308	4
365		12	max	.007	3	.022	2	.031	4	1.996e-4	4	NC	4	NC	1
366			min	-.007	2	-.02	3	0	10	-5.622e-6	10	1055.059	2	1795.553	4
367		13	max	.007	3	.02	2	.034	4	1.999e-4	4	NC	4	NC	1
368			min	-.007	2	-.017	3	0	10	-7.121e-6	10	1116.922	2	1626.297	4
369		14	max	.007	3	.016	2	.036	4	2.002e-4	4	NC	4	NC	1
370			min	-.007	2	-.013	3	0	10	-8.62e-6	10	1225.877	2	1491.302	4
371		15	max	.007	3	.01	2	.039	4	2.005e-4	4	NC	4	NC	1
372			min	-.007	2	-.009	3	0	10	-1.012e-5	10	1412.127	2	1382.814	4
373		16	max	.007	3	.004	2	.041	4	3.455e-4	4	NC	4	NC	1
374			min	-.007	2	-.003	3	0	10	-1.124e-5	10	1749.511	2	1295.351	4
375		17	max	.007	3	.003	3	.043	4	3.934e-3	4	NC	4	NC	1
376			min	-.007	2	-.005	2	0	10	-3.462e-6	10	2473.99	2	1225.067	4
377		18	max	.007	3	.01	3	.045	4	3.259e-3	2	NC	4	NC	1
378			min	-.007	2	-.014	2	0	10	-1.715e-3	3	4791.451	2	1168.889	4
379		19	max	.007	3	.018	3	.047	4	6.575e-3	2	NC	1	NC	1
380			min	-.007	2	-.024	2	0	9	-3.529e-3	3	NC	1	1125.989	4
381	M5	1	max	.02	3	.068	3	.005	5	1.678e-5	4	NC	1	NC	1
382			min	-.023	2	-.057	2	0	9	7.835e-8	11	NC	1	NC	1
383		2	max	.02	3	.039	3	.006	5	1.204e-4	3	NC	4	NC	1
384			min	-.023	2	-.032	2	0	9	-7.772e-6	9	1647.339	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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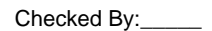
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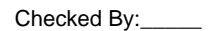
Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.02	3	.012	3	.008	5	2.295e-4	3	NC	5	NC	1
386		min	-.023	2	-.008	2	0	9	-1.546e-5	9	854.9	3	NC	1
387	4	max	.02	3	.013	2	.01	5	2.231e-4	3	NC	5	NC	1
388		min	-.023	2	-.011	3	0	9	-1.457e-5	9	611.561	3	NC	1
389	5	max	.02	3	.031	2	.013	5	2.168e-4	3	NC	5	NC	1
390		min	-.023	2	-.03	3	0	9	-1.369e-5	9	495.987	3	NC	1
391	6	max	.02	3	.046	2	.015	5	2.164e-4	5	NC	5	NC	1
392		min	-.023	2	-.045	3	0	9	-1.28e-5	9	431.912	3	9175.906	3
393	7	max	.02	3	.058	2	.018	5	2.219e-4	5	NC	5	NC	1
394		min	-.023	2	-.056	3	0	9	-1.191e-5	9	385.947	2	8733.874	3
395	8	max	.02	3	.067	2	.021	5	2.274e-4	5	NC	5	NC	1
396		min	-.023	2	-.063	3	0	9	-1.102e-5	9	356.397	2	8649.932	3
397	9	max	.02	3	.073	2	.024	4	2.329e-4	5	NC	5	NC	1
398		min	-.023	2	-.068	3	0	9	-1.013e-5	9	338.903	2	8858.428	3
399	10	max	.02	3	.076	2	.027	4	2.384e-4	5	NC	5	NC	1
400		min	-.023	2	-.068	3	0	9	-9.245e-6	9	330.821	2	9353.999	3
401	11	max	.02	3	.074	2	.03	4	2.439e-4	5	NC	5	NC	1
402		min	-.023	2	-.066	3	0	9	-8.357e-6	9	331.199	2	NC	1
403	12	max	.02	3	.07	2	.033	4	2.495e-4	4	NC	5	NC	1
404		min	-.023	2	-.06	3	0	9	-7.469e-6	9	340.422	2	NC	1
405	13	max	.019	3	.061	2	.035	4	2.554e-4	4	NC	5	NC	1
406		min	-.023	2	-.052	3	0	9	-6.58e-6	9	360.413	2	NC	1
407	14	max	.019	3	.048	2	.038	4	2.614e-4	4	NC	5	NC	1
408		min	-.023	2	-.041	3	0	9	-5.692e-6	9	395.618	2	NC	1
409	15	max	.019	3	.032	2	.04	4	2.673e-4	4	NC	5	NC	1
410		min	-.023	2	-.026	3	0	9	-4.804e-6	9	455.789	2	NC	1
411	16	max	.019	3	.011	2	.042	4	4.154e-4	4	NC	5	NC	1
412		min	-.023	2	-.01	3	0	9	-4.52e-6	9	564.764	2	NC	1
413	17	max	.019	3	.009	3	.044	4	3.949e-3	4	NC	5	NC	1
414		min	-.023	2	-.014	2	0	9	-1.863e-5	9	798.701	2	NC	1
415	18	max	.019	3	.03	3	.046	4	2.029e-3	4	NC	4	NC	1
416		min	-.023	2	-.044	2	0	9	-9.569e-6	9	1547.126	2	NC	1
417	19	max	.019	3	.052	3	.047	4	6.226e-6	5	NC	1	NC	1
418		min	-.023	2	-.075	2	0	9	-1.422e-6	3	NC	1	NC	1
419	M9	1	max	.007	.022	3	.005	5	6.805e-3	3	NC	1	NC	1
420		min	-.007	2	-.019	2	0	9	-4.721e-3	2	NC	1	NC	1
421	2	max	.007	3	.012	3	.004	4	3.361e-3	3	NC	4	NC	1
422		min	-.007	2	-.01	2	0	10	-2.335e-3	2	5050.524	3	NC	1
423	3	max	.007	3	.003	3	.004	4	7.335e-5	1	NC	4	NC	1
424		min	-.007	2	-.003	2	0	10	-3.031e-5	5	2619.248	3	NC	1
425	4	max	.007	3	.004	2	.005	4	5.851e-5	1	NC	4	NC	1
426		min	-.007	2	-.004	3	-.001	3	3.229e-5	5	1870.676	3	NC	1
427	5	max	.007	3	.01	2	.006	4	4.368e-5	1	NC	4	NC	1
428		min	-.007	2	-.01	3	-.002	3	-3.549e-5	3	1514.54	3	9783.442	3
429	6	max	.007	3	.015	2	.008	4	2.884e-5	1	NC	4	NC	1
430		min	-.007	2	-.015	3	-.003	3	-4.354e-5	3	1316.555	3	8526.611	3
431	7	max	.007	3	.019	2	.01	4	1.4e-5	1	NC	4	NC	1
432		min	-.007	2	-.019	3	-.004	3	-5.16e-5	3	1196.168	2	7806.666	3
433	8	max	.007	3	.022	2	.012	4	-2.295e-7	10	NC	4	NC	1
434		min	-.007	2	-.021	3	-.004	3	-5.965e-5	3	1104.749	2	6257.127	4
435	9	max	.007	3	.024	2	.015	4	1.261e-6	10	NC	4	NC	1
436		min	-.007	2	-.023	3	-.005	3	-6.77e-5	3	1050.623	2	4624.034	4
437	10	max	.007	3	.024	2	.017	4	2.752e-6	10	NC	4	NC	1
438		min	-.007	2	-.023	3	-.005	3	-7.575e-5	3	1025.619	2	3597.399	4
439	11	max	.007	3	.024	2	.021	5	4.243e-6	10	NC	4	NC	1
440		min	-.007	2	-.022	3	-.005	3	-8.381e-5	3	1026.797	2	2907.893	4
441	12	max	.007	3	.022	2	.024	5	5.734e-6	10	NC	4	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-0.007	2	-.02	3	-0.004	3	-9.186e-5	3	1055.346	2	2415.793	5
443		13	max	.007	3	.02	2	.027	5	7.225e-6	10	NC	4	NC	1
444			min	-0.007	2	-.017	3	-.004	3	-9.991e-5	3	1117.233	2	2054.594	5
445		14	max	.007	3	.016	2	.031	5	8.716e-6	10	NC	4	NC	1
446			min	-0.007	2	-.014	3	-.003	3	-1.08e-4	3	1226.224	2	1785.825	5
447		15	max	.007	3	.01	2	.034	5	1.021e-5	10	NC	4	NC	1
448			min	-0.007	2	-.009	3	-.003	3	-1.16e-4	3	1412.529	2	1581.044	5
449		16	max	.007	3	.004	2	.037	5	9.804e-5	5	NC	4	NC	1
450			min	-0.007	2	-.003	3	-.002	3	-1.172e-4	3	1750.005	2	1422.237	5
451		17	max	.007	3	.003	3	.041	5	3.922e-3	4	NC	4	NC	1
452			min	-0.007	2	-.005	2	-.001	1	-4.946e-5	9	2474.638	2	1297.421	5
453		18	max	.007	3	.01	3	.044	5	1.944e-3	5	NC	4	NC	1
454			min	-0.007	2	-.014	2	0	9	-3.259e-3	2	4792.665	2	1195.534	4
455		19	max	.007	3	.018	3	.047	4	3.526e-3	3	NC	1	NC	1
456			min	-0.007	2	-.024	2	0	9	-6.575e-3	2	NC	1	1112.034	4
457	M13	1	max	0	9	.022	3	.007	3	3.594e-3	3	NC	1	NC	1
458			min	-0.005	5	-.019	2	-.007	2	-3.088e-3	2	NC	1	NC	1
459		2	max	0	9	.052	3	.005	3	4.427e-3	3	NC	4	NC	1
460			min	-0.005	5	-.04	2	-.007	2	-3.807e-3	2	2732.871	3	NC	1
461		3	max	0	9	.079	3	.005	3	5.259e-3	3	NC	4	NC	1
462			min	-0.005	5	-.058	2	-.007	2	-4.525e-3	2	1476.257	3	NC	1
463		4	max	0	9	.097	3	.006	3	6.092e-3	3	NC	4	NC	1
464			min	-0.005	5	-.072	2	-.008	2	-5.243e-3	2	1114.718	3	NC	1
465		5	max	0	9	.106	3	.008	3	6.925e-3	3	NC	4	NC	1
466			min	-0.005	5	-.079	2	-.01	2	-5.961e-3	2	993.686	3	NC	1
467		6	max	0	9	.106	3	.01	3	7.758e-3	3	NC	4	NC	1
468			min	-0.005	5	-.08	2	-.013	2	-6.68e-3	2	994.828	3	NC	1
469		7	max	0	9	.099	3	.013	3	8.591e-3	3	NC	4	NC	1
470			min	-0.005	5	-.076	2	-.016	2	-7.398e-3	2	1096.122	3	NC	1
471		8	max	0	9	.086	3	.015	3	9.424e-3	3	NC	4	NC	1
472			min	-0.005	5	-.068	2	-.019	2	-8.116e-3	2	1307.133	3	7222.954	2
473		9	max	0	9	.074	3	.018	3	1.026e-2	3	NC	4	NC	1
474			min	-0.005	5	-.061	2	-.022	2	-8.834e-3	2	1614.45	3	5887.846	2
475		10	max	0	9	.068	3	.02	3	1.109e-2	3	NC	4	NC	4
476			min	-0.005	5	-.057	2	-.023	2	-9.553e-3	2	1817.769	3	5458.303	2
477		11	max	0	9	.074	3	.021	3	1.026e-2	3	NC	4	NC	1
478			min	-0.005	5	-.061	2	-.022	2	-8.834e-3	2	1614.449	3	5855.201	3
479		12	max	0	9	.086	3	.022	3	9.428e-3	3	NC	4	NC	1
480			min	-0.005	5	-.068	2	-.019	2	-8.116e-3	2	1307.132	3	5762.353	3
481		13	max	0	9	.099	3	.021	3	8.598e-3	3	NC	4	NC	1
482			min	-0.005	5	-.076	2	-.016	2	-7.398e-3	2	1096.121	3	6108.336	3
483		14	max	0	9	.107	3	.019	3	7.767e-3	3	NC	4	NC	1
484			min	-0.005	5	-.08	2	-.013	2	-6.68e-3	2	994.827	3	6957.784	3
485		15	max	0	9	.107	3	.017	3	6.937e-3	3	NC	4	NC	1
486			min	-0.005	5	-.079	2	-.01	2	-5.962e-3	2	993.685	3	8595.279	3
487		16	max	0	9	.098	3	.014	3	6.106e-3	3	NC	4	NC	1
488			min	-0.005	5	-.072	2	-.008	2	-5.243e-3	2	1114.717	3	NC	1
489		17	max	0	9	.079	3	.011	3	5.276e-3	3	NC	4	NC	1
490			min	-0.005	5	-.058	2	-.007	2	-4.525e-3	2	1476.256	3	NC	1
491		18	max	0	9	.053	3	.009	3	4.445e-3	3	NC	4	NC	1
492			min	-0.005	5	-.04	2	-.007	2	-3.807e-3	2	2732.87	3	NC	1
493		19	max	0	9	.022	3	.007	3	3.614e-3	3	NC	1	NC	1
494			min	-0.005	5	-.019	2	-.007	2	-3.089e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.018	3	.007	3	3.824e-3	2	NC	1	NC	1
496			min	-.047	4	-.024	2	-.007	2	-2.818e-3	3	NC	1	NC	1
497		2	max	0	9	.034	3	.009	3	4.715e-3	2	NC	4	NC	1
498			min	-.047	4	-.054	2	-.007	2	-3.434e-3	3	2821.419	2	NC	1







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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

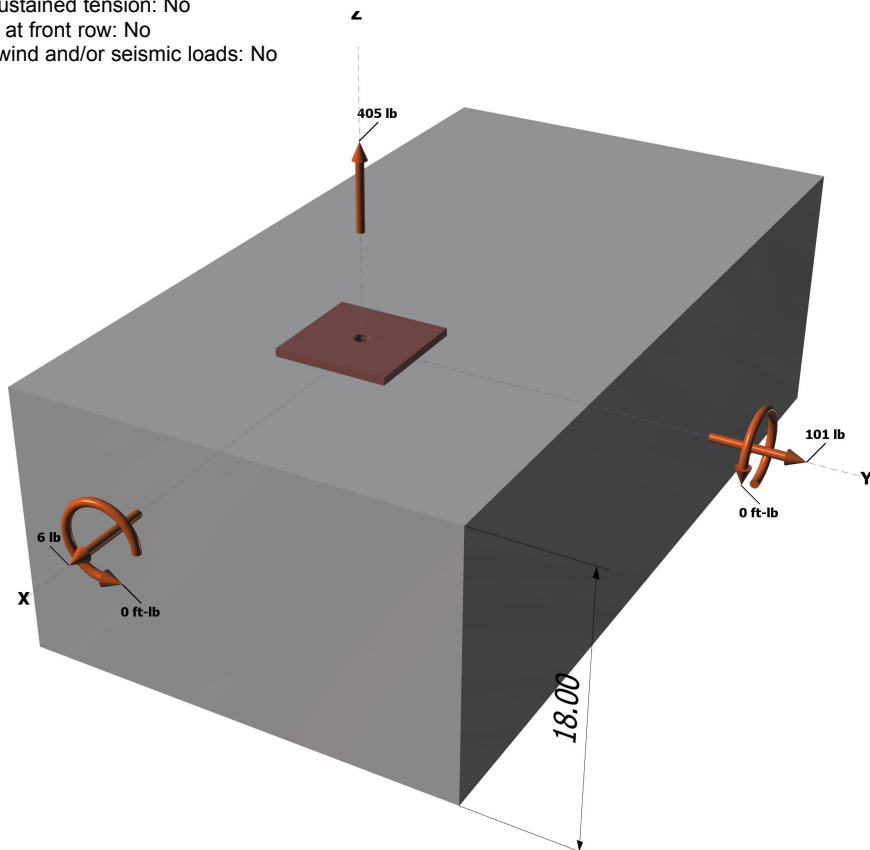
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

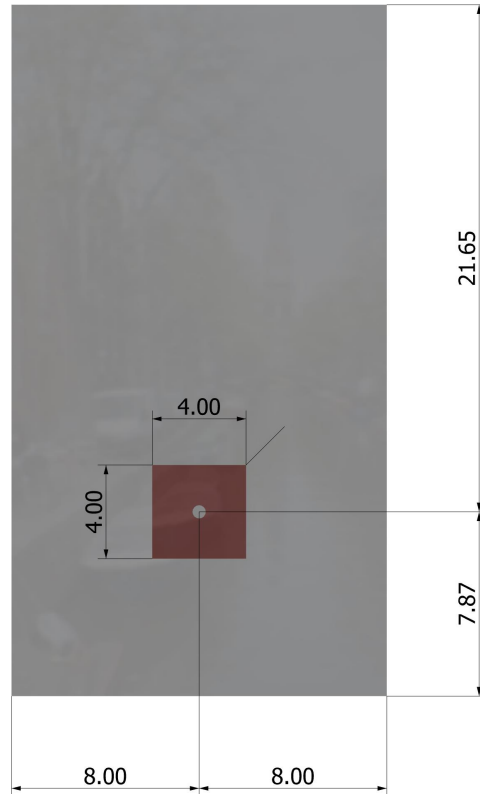
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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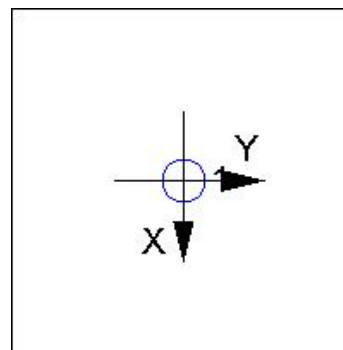
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
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E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

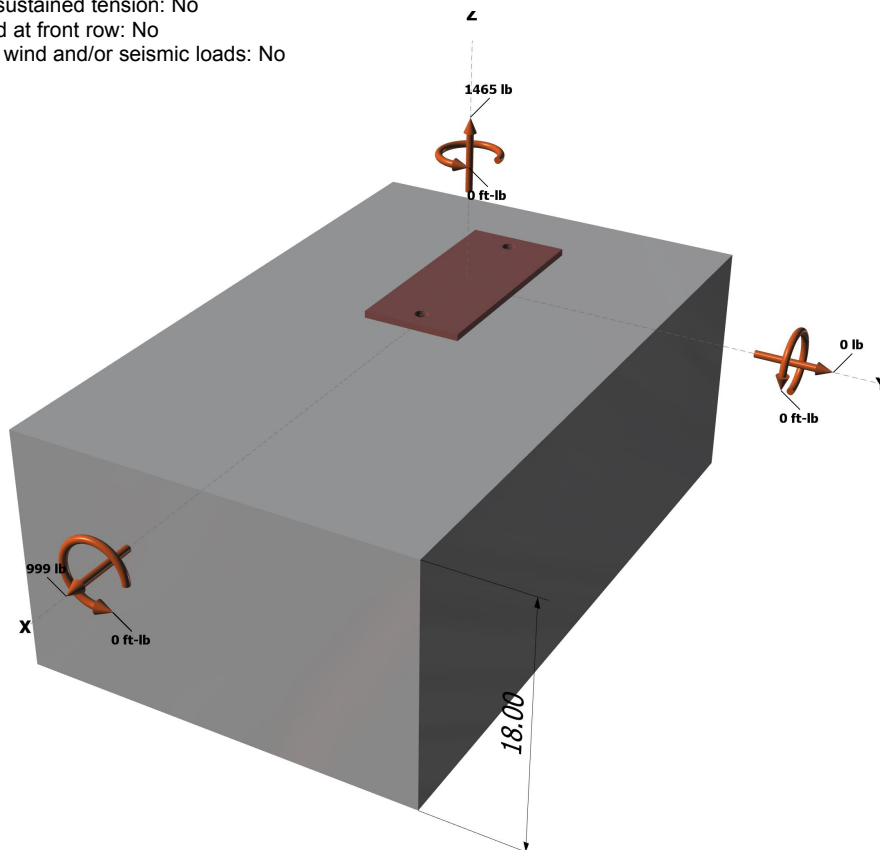
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

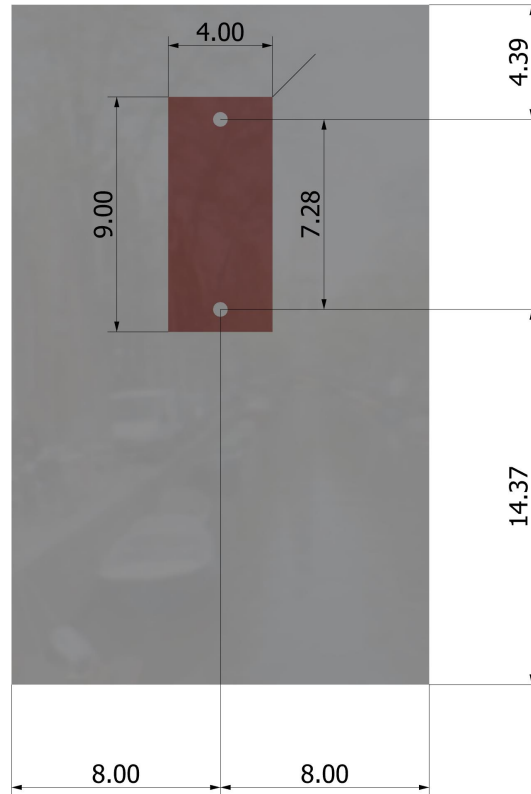
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1465
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.