

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	100 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 15.70 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad \text{(ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad \text{(ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

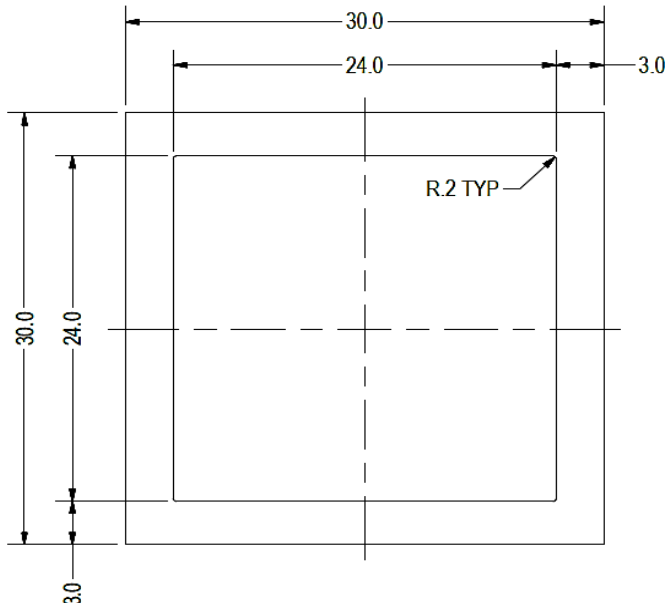
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

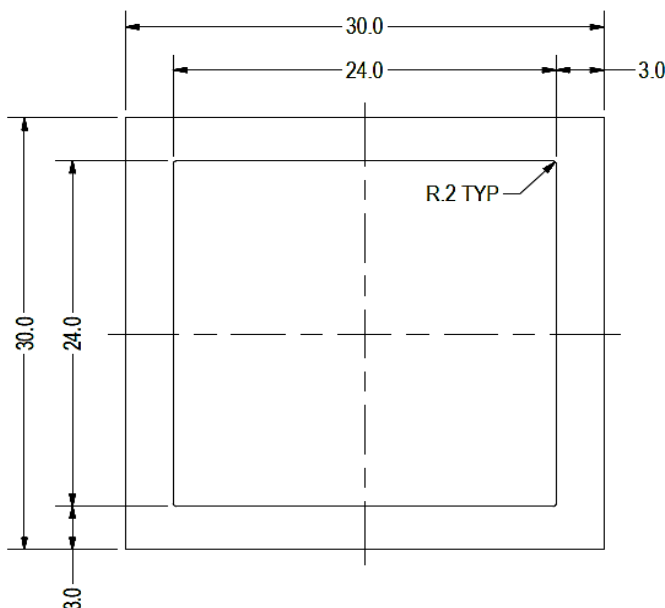
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.039 k-ft
$P_n$ =	0.199 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>11%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.628 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.799 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>16%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.005 k-ft
$P_n$ =	0.209 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>13%</b>



A cross brace kit is required every 15 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

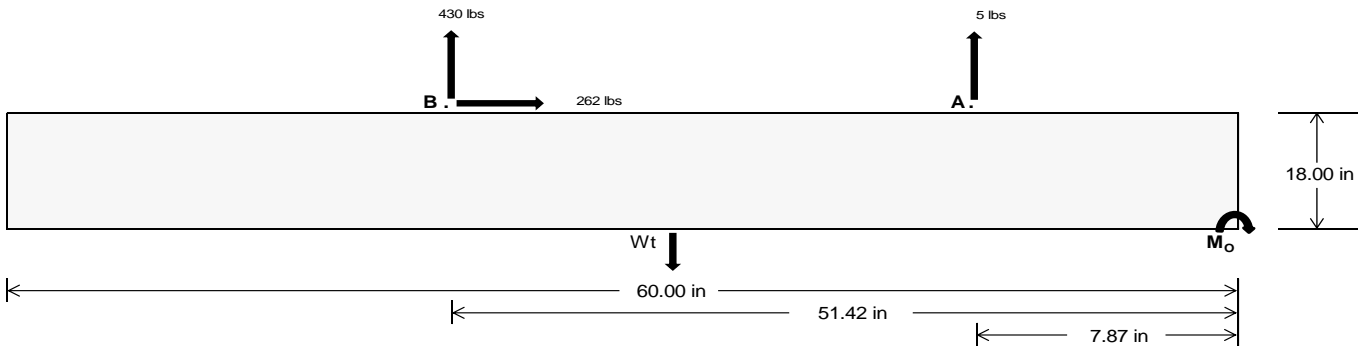
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>25.41</u>	<u>1789.40</u>	k
Compressive Load =	<u>1250.56</u>	<u>1265.18</u>	k
Lateral Load =	<u>31.93</u>	<u>1090.59</u>	k
Moment (Weak Axis) =	<u>0.05</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 26843.1$  in-lbs  
Resisting Force Required = 894.77 lbs  
S.F. = 1.67  
Weight Required = 1491.28 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 262.07 lbs  
Friction = 0.4  
Weight Required = 655.17 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 262.07 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
22 in 23 in 24 in 25 in  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$  1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	457 lbs	457 lbs	457 lbs	457 lbs	414 lbs	414 lbs	414 lbs	414 lbs	612 lbs	612 lbs	612 lbs	612 lbs	-10 lbs	-10 lbs	-10 lbs	-10 lbs
$F_B$	317 lbs	317 lbs	317 lbs	317 lbs	542 lbs	542 lbs	542 lbs	542 lbs	615 lbs	615 lbs	615 lbs	615 lbs	-859 lbs	-859 lbs	-859 lbs	-859 lbs
$F_V$	52 lbs	52 lbs	52 lbs	52 lbs	474 lbs	474 lbs	474 lbs	474 lbs	390 lbs	390 lbs	390 lbs	390 lbs	-524 lbs	-524 lbs	-524 lbs	-524 lbs
$P_{total}$	2768 lbs	2858 lbs	2949 lbs	3039 lbs	2950 lbs	3041 lbs	3131 lbs	3222 lbs	3220 lbs	3311 lbs	3401 lbs	3492 lbs	327 lbs	381 lbs	436 lbs	490 lbs
$M$	353 lbs-ft	353 lbs-ft	353 lbs-ft	353 lbs-ft	507 lbs-ft	507 lbs-ft	507 lbs-ft	507 lbs-ft	615 lbs-ft	615 lbs-ft	615 lbs-ft	615 lbs-ft	728 lbs-ft	728 lbs-ft	728 lbs-ft	728 lbs-ft
$e$	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.23 ft	1.91 ft	1.67 ft	1.49 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	255.7 psf	254.1 psf	252.5 psf	251.1 psf	255.4 psf	253.8 psf	252.3 psf	250.9 psf	270.8 psf	268.4 psf	266.3 psf	264.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	348.1 psf	342.4 psf	337.2 psf	332.4 psf	388.2 psf	380.7 psf	373.9 psf	367.7 psf	431.8 psf	422.5 psf	414.0 psf	406.1 psf	436.1 psf	224.8 psf	175.3 psf	154.7 psf

Maximum Bearing Pressure = 436 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

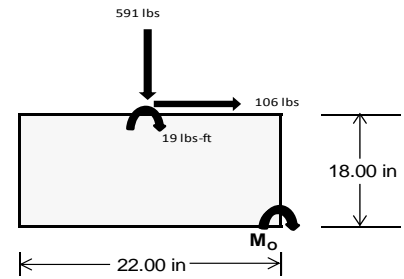
### Overturning Check

$M_o = 363.0 \text{ ft-lbs}$   
 Resisting Force Required = 396.05 lbs  
 S.F. = 1.67  
 Weight Required = 660.08 lbs  
 Minimum Width = 22 in in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	131 lbs	109 lbs	71 lbs	273 lbs	591 lbs	226 lbs	82 lbs	-16 lbs	24 lbs
$F_v$	17 lbs	141 lbs	18 lbs	12 lbs	106 lbs	14 lbs	18 lbs	141 lbs	18 lbs
$P_{total}$	2600 lbs	2577 lbs	2539 lbs	2622 lbs	2940 lbs	2576 lbs	804 lbs	706 lbs	746 lbs
$M$	50 lbs-ft	237 lbs-ft	53 lbs-ft	33 lbs-ft	179 lbs-ft	41 lbs-ft	51 lbs-ft	237 lbs-ft	53 lbs-ft
$e$	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.06 ft	0.34 ft	0.07 ft
$L/6$	0.31 ft	1.65 ft	1.79 ft	1.81 ft	1.71 ft	1.80 ft	1.71 ft	1.16 ft	1.69 ft
$f_{min}$	265.7 sqft	196.4 sqft	258.2 sqft	274.4 sqft	257.0 sqft	266.5 sqft	69.6 sqft	-7.6 sqft	62.6 sqft
$f_{max}$	301.5 psf	365.9 psf	295.8 psf	297.8 psf	384.5 psf	295.5 psf	105.7 psf	161.7 psf	100.2 psf



Maximum Bearing Pressure = 385 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



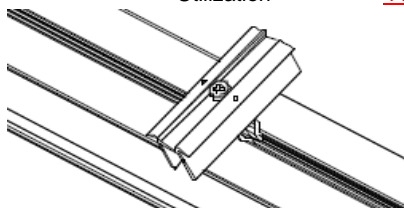
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

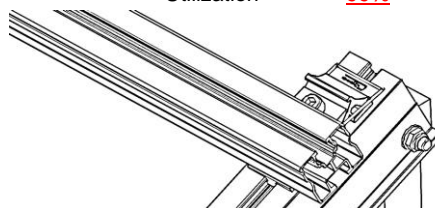
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.540 k
Allowable Uplift =	1.214 k
Utilization =	<u>44%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.103 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

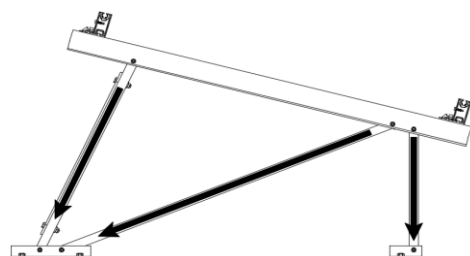
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.962 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

#### Diagonal Strut

Maximum Axial Load =	0.628 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.163 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.209 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.082 in
	<u>0.082 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.255$$

$$179.672$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.255$$

$$186.579$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.9$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.0 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.234 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.10 \\
 &23.4092 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.10 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.443 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

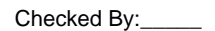
$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	250.388	1	-.016	15	.345	1	0	10	0	4	0	15
30			min	-361.65	3	-.073	3	-.454	5	0	1	0	3	0	6
31		16	max	250.514	1	-.028	15	.345	1	0	10	0	4	0	15
32			min	-361.556	3	-.115	4	-.569	5	0	1	0	3	0	6
33		17	max	250.64	1	-.04	15	.345	1	0	10	0	1	0	15
34			min	-361.461	3	-.166	4	-.683	5	0	1	0	3	0	6
35		18	max	250.766	1	-.052	15	.345	1	0	10	0	1	0	15
36			min	-361.367	3	-.217	4	-.798	5	0	1	0	3	0	6
37		19	max	250.892	1	-.064	15	.345	1	0	10	0	1	0	15
38			min	-361.273	3	-.268	4	-.912	5	0	1	0	3	0	6
39	M3	1	max	167.229	2	1.756	6	-.03	10	0	5	.001	1	0	6
40			min	-173.997	3	.412	15	-1.375	4	0	1	0	10	0	15
41		2	max	167.159	2	1.579	6	-.03	10	0	5	.001	1	0	2
42			min	-174.049	3	.371	15	-1.242	4	0	1	0	10	0	12
43		3	max	167.09	2	1.402	6	-.03	10	0	5	.001	1	0	2
44			min	-174.101	3	.329	15	-1.108	4	0	1	0	5	0	3
45		4	max	167.021	2	1.226	6	-.03	10	0	5	0	1	0	15
46			min	-174.153	3	.287	15	-.974	4	0	1	0	5	0	4
47		5	max	166.952	2	1.049	6	-.03	10	0	5	0	1	0	15
48			min	-174.205	3	.246	15	-.841	4	0	1	0	5	0	4
49		6	max	166.882	2	.872	6	-.03	10	0	5	0	1	0	15
50			min	-174.257	3	.204	15	-.707	4	0	1	0	5	0	4
51		7	max	166.813	2	.695	6	-.03	10	0	5	0	1	0	15
52			min	-174.309	3	.163	15	-.573	4	0	1	0	5	0	4
53		8	max	166.744	2	.518	6	-.03	10	0	5	0	1	0	15
54			min	-174.361	3	.121	15	-.44	4	0	1	0	5	-.001	4
55		9	max	166.674	2	.341	6	-.03	10	0	5	0	1	0	15
56			min	-174.413	3	.08	15	-.353	1	0	1	0	5	-.001	4
57		10	max	166.605	2	.165	6	-.03	10	0	5	0	1	0	15
58			min	-174.465	3	.038	15	-.353	1	0	1	0	5	-.001	4
59		11	max	166.536	2	.017	2	.04	5	0	5	0	1	0	15
60			min	-174.517	3	-.038	3	-.353	1	0	1	0	5	-.001	4
61		12	max	166.466	2	-.045	15	.174	5	0	5	0	1	0	15
62			min	-174.569	3	-.189	4	-.353	1	0	1	0	5	-.001	4
63		13	max	166.397	2	-.087	15	.307	5	0	5	0	1	0	15
64			min	-174.621	3	-.366	4	-.353	1	0	1	0	5	-.001	4
65		14	max	166.328	2	-.128	15	.441	5	0	5	0	1	0	15
66			min	-174.673	3	-.543	4	-.353	1	0	1	0	5	-.001	4
67		15	max	166.258	2	-.17	15	.575	5	0	5	0	1	0	15
68			min	-174.725	3	-.72	4	-.353	1	0	1	0	5	0	4
69		16	max	166.189	2	-.211	15	.708	5	0	5	0	1	0	15
70			min	-174.777	3	-.897	4	-.353	1	0	1	0	5	0	4
71		17	max	166.12	2	-.253	15	.842	5	0	5	0	12	0	15
72			min	-174.829	3	-1.073	4	-.353	1	0	1	0	4	0	4
73		18	max	166.05	2	-.294	15	.976	5	0	5	0	12	0	15
74			min	-174.881	3	-1.25	4	-.353	1	0	1	0	1	0	4
75		19	max	165.981	2	-.336	15	1.109	5	0	5	0	5	0	1
76			min	-174.933	3	-1.427	4	-.353	1	0	1	0	1	0	1
77	M4	1	max	359.826	1	0	1	-.124	10	0	1	0	5	0	1
78			min	3.411	12	0	1	-23.599	4	0	1	0	2	0	1
79		2	max	359.891	1	0	1	-.124	10	0	1	0	12	0	1
80			min	3.444	12	0	1	-23.655	4	0	1	-.002	4	0	1
81		3	max	359.955	1	0	1	-.124	10	0	1	0	12	0	1
82			min	3.476	12	0	1	-23.711	4	0	1	-.004	4	0	1
83		4	max	360.02	1	0	1	-.124	10	0	1	0	12	0	1
84			min	3.508	12	0	1	-23.767	4	0	1	-.006	4	0	1
85		5	max	360.085	1	0	1	-.124	10	0	1	0	10	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	3.541	12	0	1	-23.823	4	0	1	-.008	4	0	1
87		6	max	360.149	1	0	1	-.124	10	0	1	0	10	0	1
88			min	3.573	12	0	1	-23.879	4	0	1	-.011	4	0	1
89		7	max	360.214	1	0	1	-.124	10	0	1	0	10	0	1
90			min	3.605	12	0	1	-23.935	4	0	1	-.013	4	0	1
91		8	max	360.279	1	0	1	-.124	10	0	1	0	10	0	1
92			min	3.638	12	0	1	-23.991	4	0	1	-.015	4	0	1
93		9	max	360.344	1	0	1	-.124	10	0	1	0	10	0	1
94			min	3.67	12	0	1	-24.048	4	0	1	-.017	4	0	1
95		10	max	360.408	1	0	1	-.124	10	0	1	0	10	0	1
96			min	3.702	12	0	1	-24.104	4	0	1	-.019	4	0	1
97		11	max	360.473	1	0	1	-.124	10	0	1	0	10	0	1
98			min	3.735	12	0	1	-24.16	4	0	1	-.021	4	0	1
99		12	max	360.538	1	0	1	-.124	10	0	1	0	10	0	1
100			min	3.767	12	0	1	-24.216	4	0	1	-.023	4	0	1
101		13	max	360.602	1	0	1	-.124	10	0	1	0	10	0	1
102			min	3.8	12	0	1	-24.272	4	0	1	-.026	4	0	1
103		14	max	360.667	1	0	1	-.124	10	0	1	0	10	0	1
104			min	3.832	12	0	1	-24.328	4	0	1	-.028	4	0	1
105		15	max	360.732	1	0	1	-.124	10	0	1	0	10	0	1
106			min	3.864	12	0	1	-24.384	4	0	1	-.03	4	0	1
107		16	max	360.797	1	0	1	-.124	10	0	1	0	10	0	1
108			min	3.897	12	0	1	-24.44	4	0	1	-.032	4	0	1
109		17	max	360.861	1	0	1	-.124	10	0	1	0	10	0	1
110			min	3.929	12	0	1	-24.496	4	0	1	-.034	4	0	1
111		18	max	360.926	1	0	1	-.124	10	0	1	0	10	0	1
112			min	3.961	12	0	1	-24.552	4	0	1	-.037	4	0	1
113		19	max	360.991	1	0	1	-.124	10	0	1	0	10	0	1
114			min	3.994	12	0	1	-24.608	4	0	1	-.039	4	0	1
115	M6	1	max	796.293	1	.641	6	1.145	4	0	3	0	3	0	1
116			min	-1162.61	3	.143	15	-.189	3	0	5	0	9	0	1
117		2	max	796.419	1	.59	6	1.03	4	0	3	0	4	0	15
118			min	-1162.516	3	.131	15	-.189	3	0	5	0	9	0	6
119		3	max	796.545	1	.539	6	.916	4	0	3	0	4	0	15
120			min	-1162.421	3	.119	15	-.189	3	0	5	0	10	0	6
121		4	max	796.671	1	.489	2	.802	4	0	3	0	4	0	15
122			min	-1162.327	3	.107	15	-.189	3	0	5	0	10	0	6
123		5	max	796.797	1	.449	2	.687	4	0	3	0	4	0	15
124			min	-1162.233	3	.095	15	-.189	3	0	5	0	10	0	6
125		6	max	796.923	1	.409	2	.573	4	0	3	0	4	0	15
126			min	-1162.138	3	.083	12	-.189	3	0	5	0	10	0	6
127		7	max	797.049	1	.37	2	.458	4	0	3	0	4	0	15
128			min	-1162.044	3	.063	12	-.189	3	0	5	0	3	0	2
129		8	max	797.175	1	.33	2	.344	4	0	3	.001	4	0	15
130			min	-1161.949	3	.043	12	-.189	3	0	5	0	3	0	2
131		9	max	797.3	1	.29	2	.229	4	0	3	.001	4	0	15
132			min	-1161.855	3	.023	12	-.189	3	0	5	0	3	0	2
133		10	max	797.426	1	.25	2	.115	4	0	3	.001	4	0	15
134			min	-1161.761	3	-.003	3	-.189	3	0	5	0	3	0	2
135		11	max	797.552	1	.21	2	.104	1	0	3	.001	4	0	12
136			min	-1161.666	3	-.033	3	-.189	3	0	5	0	3	0	2
137		12	max	797.678	1	.17	2	.104	1	0	3	.001	4	0	12
138			min	-1161.572	3	-.063	3	-.189	3	0	5	0	3	0	2
139		13	max	797.804	1	.13	2	.104	1	0	3	.001	4	0	12
140			min	-1161.477	3	-.093	3	-.259	5	0	5	0	3	0	2
141		14	max	797.93	1	.091	2	.104	1	0	3	.001	4	0	12
142			min	-1161.383	3	-.122	3	-.374	5	0	5	0	3	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	798.056	1	.051	2	.104	1	0	3	0	4	0	12
144		min	-1161.289	3	-.152	3	-.488	5	0	5	0	3	0	2
145	16	max	798.181	1	.011	2	.104	1	0	3	0	4	0	12
146		min	-1161.194	3	-.182	3	-.603	5	0	5	0	3	0	2
147	17	max	798.307	1	-.029	2	.104	1	0	3	0	4	0	12
148		min	-1161.1	3	-.212	3	-.717	5	0	5	0	3	0	2
149	18	max	798.433	1	-.061	15	.104	1	0	3	0	4	0	3
150		min	-1161.005	3	-.242	3	-.832	5	0	5	0	3	0	2
151	19	max	798.559	1	-.073	15	.104	1	0	3	0	4	0	3
152		min	-1160.911	3	-.28	4	-.946	5	0	5	0	3	0	2
153	M7	1	max	628.404	2	1.774	.023	3	0	1	0	4	0	2
154		min	-533.426	3	.423	15	-1.329	5	0	3	0	3	0	3
155	2	max	628.334	2	1.597	4	.023	3	0	1	0	4	0	2
156		min	-533.478	3	.381	15	-1.195	5	0	3	0	3	0	3
157	3	max	628.265	2	1.42	4	.023	3	0	1	0	1	0	2
158		min	-533.53	3	.339	15	-1.062	5	0	3	0	3	0	3
159	4	max	628.196	2	1.243	4	.023	3	0	1	0	1	0	2
160		min	-533.582	3	.298	15	-.928	5	0	3	0	3	0	3
161	5	max	628.126	2	1.067	4	.023	3	0	1	0	1	0	15
162		min	-533.634	3	.256	15	-.794	5	0	3	0	5	0	3
163	6	max	628.057	2	.89	4	.023	3	0	1	0	1	0	15
164		min	-533.686	3	.215	15	-.661	5	0	3	0	5	0	3
165	7	max	627.988	2	.713	4	.023	3	0	1	0	1	0	15
166		min	-533.738	3	.173	15	-.527	5	0	3	0	5	0	6
167	8	max	627.918	2	.536	4	.023	3	0	1	0	1	0	15
168		min	-533.79	3	.132	15	-.393	5	0	3	0	5	-.001	6
169	9	max	627.849	2	.359	4	.023	3	0	1	0	1	0	15
170		min	-533.842	3	.069	12	-.259	5	0	3	0	5	-.001	6
171	10	max	627.78	2	.22	2	.023	3	0	1	0	1	0	15
172		min	-533.894	3	-.007	3	-.126	5	0	3	0	5	-.001	6
173	11	max	627.71	2	.082	2	.023	3	0	1	0	1	0	15
174		min	-533.946	3	-.11	3	-.013	2	0	3	0	5	-.001	6
175	12	max	627.641	2	-.035	15	.142	4	0	1	0	1	0	15
176		min	-533.998	3	-.213	3	-.013	2	0	3	0	5	-.001	6
177	13	max	627.572	2	-.076	15	.276	4	0	1	0	1	0	15
178		min	-534.05	3	-.349	6	-.013	2	0	3	0	5	-.001	6
179	14	max	627.503	2	-.118	15	.409	4	0	1	0	1	0	15
180		min	-534.102	3	-.525	6	-.013	2	0	3	0	5	-.001	6
181	15	max	627.433	2	-.159	15	.543	4	0	1	0	1	0	15
182		min	-534.154	3	-.702	6	-.013	2	0	3	0	5	0	6
183	16	max	627.364	2	-.201	15	.677	4	0	1	0	1	0	15
184		min	-534.206	3	-.879	6	-.013	2	0	3	0	5	0	6
185	17	max	627.295	2	-.242	15	.81	4	0	1	0	1	0	15
186		min	-534.258	3	-1.056	6	-.013	2	0	3	0	5	0	6
187	18	max	627.225	2	-.284	15	.944	4	0	1	0	1	0	15
188		min	-534.31	3	-1.233	6	-.013	2	0	3	0	5	0	6
189	19	max	627.156	2	-.326	15	1.078	4	0	1	0	1	0	1
190		min	-534.362	3	-1.41	6	-.013	2	0	3	0	3	0	1
191	M8	1	max	960.806	1	0	.636	1	0	1	0	4	0	1
192		min	-20.421	3	0	1	-23.787	4	0	1	0	1	0	1
193	2	max	960.87	1	0	1	.636	1	0	1	0	1	0	1
194		min	-20.373	3	0	1	-23.843	4	0	1	-.002	4	0	1
195	3	max	960.935	1	0	1	.636	1	0	1	0	1	0	1
196		min	-20.324	3	0	1	-23.899	4	0	1	-.004	4	0	1
197	4	max	961	1	0	1	.636	1	0	1	0	1	0	1
198		min	-20.276	3	0	1	-23.955	4	0	1	-.006	4	0	1
199	5	max	961.064	1	0	1	.636	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-20.227	3	0	1	-24.011	4	0	1	-.009	4	0	1
201		6	max	961.129	1	0	1	.636	1	0	1	0	1	0	1
202			min	-20.179	3	0	1	-24.068	4	0	1	-.011	4	0	1
203		7	max	961.194	1	0	1	.636	1	0	1	0	1	0	1
204			min	-20.13	3	0	1	-24.124	4	0	1	-.013	4	0	1
205		8	max	961.259	1	0	1	.636	1	0	1	0	1	0	1
206			min	-20.082	3	0	1	-24.18	4	0	1	-.015	4	0	1
207		9	max	961.323	1	0	1	.636	1	0	1	0	1	0	1
208			min	-20.033	3	0	1	-24.236	4	0	1	-.017	4	0	1
209		10	max	961.388	1	0	1	.636	1	0	1	0	1	0	1
210			min	-19.984	3	0	1	-24.292	4	0	1	-.019	4	0	1
211		11	max	961.453	1	0	1	.636	1	0	1	0	1	0	1
212			min	-19.936	3	0	1	-24.348	4	0	1	-.022	4	0	1
213		12	max	961.517	1	0	1	.636	1	0	1	0	1	0	1
214			min	-19.887	3	0	1	-24.404	4	0	1	-.024	4	0	1
215		13	max	961.582	1	0	1	.636	1	0	1	0	1	0	1
216			min	-19.839	3	0	1	-24.46	4	0	1	-.026	4	0	1
217		14	max	961.647	1	0	1	.636	1	0	1	0	1	0	1
218			min	-19.79	3	0	1	-24.516	4	0	1	-.028	4	0	1
219		15	max	961.712	1	0	1	.636	1	0	1	0	1	0	1
220			min	-19.742	3	0	1	-24.572	4	0	1	-.03	4	0	1
221		16	max	961.776	1	0	1	.636	1	0	1	0	1	0	1
222			min	-19.693	3	0	1	-24.628	4	0	1	-.032	4	0	1
223		17	max	961.841	1	0	1	.636	1	0	1	0	1	0	1
224			min	-19.645	3	0	1	-24.684	4	0	1	-.035	4	0	1
225		18	max	961.906	1	0	1	.636	1	0	1	0	1	0	1
226			min	-19.596	3	0	1	-24.741	4	0	1	-.037	4	0	1
227		19	max	961.97	1	0	1	.636	1	0	1	.001	1	0	1
228			min	-19.548	3	0	1	-24.797	4	0	1	-.039	4	0	1
229	M10	1	max	251.295	1	.682	4	1.295	5	0	1	0	1	0	1
230			min	-327.321	3	.173	15	-.145	1	-.001	5	0	3	0	1
231		2	max	251.421	1	.631	4	1.18	5	0	1	0	1	0	15
232			min	-327.227	3	.161	15	-.145	1	-.001	5	0	3	0	4
233		3	max	251.547	1	.58	4	1.066	5	0	1	0	4	0	15
234			min	-327.133	3	.149	15	-.145	1	-.001	5	0	3	0	4
235		4	max	251.673	1	.529	4	.951	5	0	1	0	4	0	15
236			min	-327.038	3	.137	15	-.145	1	-.001	5	0	3	0	4
237		5	max	251.799	1	.478	4	.837	5	0	1	0	4	0	15
238			min	-326.944	3	.125	15	-.145	1	-.001	5	0	3	0	4
239		6	max	251.925	1	.427	4	.722	5	0	1	0	4	0	15
240			min	-326.849	3	.113	15	-.145	1	-.001	5	0	3	0	4
241		7	max	252.051	1	.375	4	.608	5	0	1	0	4	0	15
242			min	-326.755	3	.101	15	-.145	1	-.001	5	0	3	0	4
243		8	max	252.177	1	.324	4	.494	5	0	1	.001	4	0	15
244			min	-326.661	3	.089	15	-.145	1	-.001	5	0	3	0	4
245		9	max	252.302	1	.273	4	.379	5	0	1	.001	4	0	15
246			min	-326.566	3	.077	15	-.145	1	-.001	5	0	3	0	4
247		10	max	252.428	1	.222	4	.265	5	0	1	.001	4	0	15
248			min	-326.472	3	.065	15	-.145	1	-.001	5	0	3	0	4
249		11	max	252.554	1	.171	4	.15	5	0	1	.001	4	0	15
250			min	-326.377	3	.048	12	-.145	1	-.001	5	0	3	0	4
251		12	max	252.68	1	.12	4	.036	5	0	1	.001	4	0	15
252			min	-326.283	3	.028	12	-.145	1	-.001	5	0	3	0	4
253		13	max	252.806	1	.069	4	-.005	12	0	1	.001	4	0	15
254			min	-326.189	3	.009	12	-.145	1	-.001	5	0	3	0	4
255		14	max	252.932	1	.024	5	-.005	12	0	1	.001	5	0	15
256			min	-326.094	3	-.021	9	-.215	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	253.058	1	.006	5	-.005	12	0	1	.001	5	0	15
258			min	-326	3	-.054	9	-.33	4	-.001	5	0	3	0	4
259		16	max	253.184	1	-.008	15	-.005	12	0	1	.001	5	0	15
260			min	-325.905	3	-.087	9	-.444	4	-.001	5	0	3	0	4
261		17	max	253.309	1	-.02	15	-.005	12	0	1	0	5	0	15
262			min	-325.811	3	-.137	6	-.558	4	-.001	5	0	1	0	4
263		18	max	253.435	1	-.032	15	-.005	12	0	1	0	5	0	15
264			min	-325.717	3	-.188	6	-.673	4	-.001	5	0	1	0	4
265		19	max	253.561	1	-.044	15	-.005	12	0	1	0	5	0	15
266			min	-325.622	3	-.239	6	-.787	4	-.001	5	0	1	0	4
267	M11	1	max	166.854	2	1.747	6	.388	1	.001	4	0	5	0	6
268			min	-174.65	3	.406	15	-1.23	5	0	10	-.001	1	0	15
269		2	max	166.785	2	1.57	6	.388	1	.001	4	0	5	0	2
270			min	-174.702	3	.364	15	-1.096	5	0	10	-.001	1	0	3
271		3	max	166.715	2	1.393	6	.388	1	.001	4	0	5	0	2
272			min	-174.754	3	.322	15	-.963	5	0	10	0	1	0	3
273		4	max	166.646	2	1.216	6	.388	1	.001	4	0	3	0	15
274			min	-174.806	3	.281	15	-.829	5	0	10	0	1	0	4
275		5	max	166.577	2	1.039	6	.388	1	.001	4	0	3	0	15
276			min	-174.858	3	.239	15	-.695	5	0	10	0	1	0	4
277		6	max	166.508	2	.863	6	.388	1	.001	4	0	3	0	15
278			min	-174.91	3	.198	15	-.562	5	0	10	0	1	0	4
279		7	max	166.438	2	.686	6	.388	1	.001	4	0	3	0	15
280			min	-174.962	3	.156	15	-.428	5	0	10	0	1	0	4
281		8	max	166.369	2	.509	6	.388	1	.001	4	0	3	0	15
282			min	-175.014	3	.115	15	-.294	5	0	10	0	1	-.001	4
283		9	max	166.3	2	.332	6	.388	1	.001	4	0	3	0	15
284			min	-175.066	3	.073	15	-.161	5	0	10	0	1	-.001	4
285		10	max	166.23	2	.155	6	.388	1	.001	4	0	3	0	15
286			min	-175.118	3	.031	15	-.027	5	0	10	0	1	-.001	4
287		11	max	166.161	2	.017	2	.388	1	.001	4	0	3	0	15
288			min	-175.17	3	-.054	3	-.026	3	0	10	0	4	-.001	4
289		12	max	166.092	2	-.052	15	.388	1	.001	4	0	3	0	15
290			min	-175.222	3	-.199	4	-.026	3	0	10	0	4	-.001	4
291		13	max	166.022	2	-.093	15	.461	4	.001	4	0	3	0	15
292			min	-175.274	3	-.376	4	-.026	3	0	10	0	4	-.001	4
293		14	max	165.953	2	-.135	15	.594	4	.001	4	0	3	0	15
294			min	-175.326	3	-.553	4	-.026	3	0	10	0	4	-.001	4
295		15	max	165.884	2	-.176	15	.728	4	.001	4	0	3	0	15
296			min	-175.378	3	-.729	4	-.026	3	0	10	0	2	0	4
297		16	max	165.814	2	-.218	15	.862	4	.001	4	0	3	0	15
298			min	-175.43	3	-.906	4	-.026	3	0	10	0	10	0	4
299		17	max	165.745	2	-.259	15	.995	4	.001	4	0	4	0	15
300			min	-175.482	3	-1.083	4	-.026	3	0	10	0	10	0	4
301		18	max	165.676	2	-.301	15	1.129	4	.001	4	0	4	0	15
302			min	-175.534	3	-1.26	4	-.026	3	0	10	0	10	0	4
303		19	max	165.606	2	-.343	15	1.263	4	.001	4	0	4	0	1
304			min	-175.586	3	-1.437	4	-.026	3	0	10	0	10	0	1
305	M12	1	max	359.58	1	0	1	2.319	1	0	1	0	4	0	1
306			min	3.772	12	0	1	-21.786	5	0	1	0	3	0	1
307		2	max	359.645	1	0	1	2.319	1	0	1	0	1	0	1
308			min	3.805	12	0	1	-21.842	5	0	1	-.002	5	0	1
309		3	max	359.71	1	0	1	2.319	1	0	1	0	1	0	1
310			min	3.837	12	0	1	-21.898	5	0	1	-.004	5	0	1
311		4	max	359.774	1	0	1	2.319	1	0	1	0	1	0	1
312			min	3.869	12	0	1	-21.954	5	0	1	-.006	5	0	1
313		5	max	359.839	1	0	1	2.319	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	3.902	12	0	1	-22.01	5	0	1	-.008	5	0	1
315		6	max	359.904	1	0	1	2.319	1	0	1	.001	1	0	1
316			min	3.934	12	0	1	-22.067	5	0	1	-.01	5	0	1
317		7	max	359.969	1	0	1	2.319	1	0	1	.001	1	0	1
318			min	3.966	12	0	1	-22.123	5	0	1	-.012	5	0	1
319		8	max	360.033	1	0	1	2.319	1	0	1	.001	1	0	1
320			min	3.999	12	0	1	-22.179	5	0	1	-.014	5	0	1
321		9	max	360.098	1	0	1	2.319	1	0	1	.002	1	0	1
322			min	4.031	12	0	1	-22.235	5	0	1	-.016	5	0	1
323		10	max	360.163	1	0	1	2.319	1	0	1	.002	1	0	1
324			min	4.063	12	0	1	-22.291	5	0	1	-.018	5	0	1
325		11	max	360.227	1	0	1	2.319	1	0	1	.002	1	0	1
326			min	4.096	12	0	1	-22.347	5	0	1	-.02	5	0	1
327		12	max	360.292	1	0	1	2.319	1	0	1	.002	1	0	1
328			min	4.128	12	0	1	-22.403	5	0	1	-.022	5	0	1
329		13	max	360.357	1	0	1	2.319	1	0	1	.003	1	0	1
330			min	4.16	12	0	1	-22.459	5	0	1	-.024	5	0	1
331		14	max	360.421	1	0	1	2.319	1	0	1	.003	1	0	1
332			min	4.193	12	0	1	-22.515	5	0	1	-.026	5	0	1
333		15	max	360.486	1	0	1	2.319	1	0	1	.003	1	0	1
334			min	4.225	12	0	1	-22.571	5	0	1	-.028	5	0	1
335		16	max	360.551	1	0	1	2.319	1	0	1	.003	1	0	1
336			min	4.257	12	0	1	-22.627	5	0	1	-.03	5	0	1
337		17	max	360.616	1	0	1	2.319	1	0	1	.003	1	0	1
338			min	4.29	12	0	1	-22.683	5	0	1	-.032	5	0	1
339		18	max	360.68	1	0	1	2.319	1	0	1	.004	1	0	1
340			min	4.322	12	0	1	-22.739	5	0	1	-.034	5	0	1
341		19	max	360.745	1	0	1	2.319	1	0	1	.004	1	0	1
342			min	4.355	12	0	1	-22.796	5	0	1	-.036	5	0	1
343	M1	1	max	114.735	1	341.407	3	-3.144	12	0	1	.09	1	0	2
344			min	6.198	12	-248.51	1	-45.888	1	0	3	.007	12	0	3
345		2	max	114.875	1	341.226	3	-3.144	12	0	1	.08	1	.054	1
346			min	6.268	12	-248.752	1	-45.888	1	0	3	.006	12	-.074	3
347		3	max	88.065	3	5.805	14	-3.182	12	0	12	.07	1	.107	1
348			min	-12.444	10	-23.789	2	-45.751	1	0	1	.006	10	-.147	3
349		4	max	88.17	3	5.579	9	-3.182	12	0	12	.06	1	.109	2
350			min	-12.328	10	-24.031	2	-45.751	1	0	1	.005	10	-.144	3
351		5	max	88.275	3	5.377	9	-3.182	12	0	12	.05	1	.114	2
352			min	-12.212	10	-24.273	2	-45.751	1	0	1	.004	10	-.141	3
353		6	max	88.379	3	5.175	9	-3.182	12	0	12	.04	1	.12	2
354			min	-12.095	10	-24.515	2	-45.751	1	0	1	.003	10	-.138	3
355		7	max	88.484	3	4.974	9	-3.182	12	0	12	.03	1	.125	2
356			min	-11.979	10	-24.757	2	-45.751	1	0	1	.002	10	-.135	3
357		8	max	88.589	3	4.772	9	-3.182	12	0	12	.02	1	.13	2
358			min	-11.863	10	-24.998	2	-45.751	1	0	1	.002	10	-.132	3
359		9	max	88.693	3	4.571	9	-3.182	12	0	12	.01	1	.136	2
360			min	-11.746	10	-25.24	2	-45.751	1	0	1	0	10	-.129	3
361		10	max	88.798	3	4.369	9	-3.182	12	0	12	.001	4	.141	2
362			min	-11.63	10	-25.482	2	-45.751	1	0	1	0	10	-.126	3
363		11	max	88.903	3	4.168	9	-3.182	12	0	12	0	3	.147	2
364			min	-11.514	10	-25.724	2	-45.751	1	0	1	-.01	1	-.122	3
365		12	max	89.008	3	3.966	9	-3.182	12	0	12	0	12	.153	2
366			min	-11.397	10	-25.966	2	-45.751	1	0	1	-.02	1	-.119	3
367		13	max	89.112	3	3.765	9	-3.182	12	0	12	-.001	12	.158	2
368			min	-11.281	10	-26.208	2	-45.751	1	0	1	-.03	1	-.116	3
369		14	max	89.217	3	3.563	9	-3.182	12	0	12	-.002	12	.164	2
370			min	-11.165	10	-26.449	2	-45.751	1	0	1	-.04	1	-.113	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	89.322	3	3.362	9	-3.182	12	0	12	-.003	12	.17	2
372			min	-11.048	10	-26.691	2	-45.751	1	0	1	-.049	1	-.109	3
373		16	max	91.604	2	111.555	2	-3.215	12	0	1	-.003	12	.174	2
374			min	-5.904	3	-162.416	3	-46.053	1	0	5	-.06	1	-.105	3
375		17	max	91.744	2	111.313	2	-3.215	12	0	1	-.004	12	.15	2
376			min	-5.8	3	-162.598	3	-46.053	1	0	5	-.07	1	-.069	3
377		18	max	-5.126	12	347.504	2	-3.409	12	0	3	-.005	12	.076	2
378			min	-114.857	1	-159.22	3	-47.157	1	0	2	-.08	1	-.035	3
379		19	max	-5.057	12	347.262	2	-3.409	12	0	3	-.006	12	0	2
380			min	-114.718	1	-159.401	3	-47.157	1	0	2	-.09	1	0	3
381	M5	1	max	259.302	1	1120.889	3	0	10	0	1	.037	4	0	3
382			min	6.959	12	-815.531	1	-54.465	3	0	5	0	10	0	2
383		2	max	259.442	1	1120.708	3	0	10	0	1	.032	4	.176	1
384			min	7.028	12	-815.773	1	-54.465	3	0	5	-.005	3	-.243	3
385		3	max	268.247	3	5.626	9	6.162	3	0	3	.026	4	.35	1
386			min	-48.197	10	-87.144	2	-20.11	4	0	4	-.016	3	-.481	3
387		4	max	268.352	3	5.424	9	6.162	3	0	3	.022	4	.358	1
388			min	-48.081	10	-87.386	2	-19.868	4	0	4	-.015	3	-.47	3
389		5	max	268.457	3	5.223	9	6.162	3	0	3	.018	4	.377	2
390			min	-47.965	10	-87.628	2	-19.626	4	0	4	-.014	3	-.459	3
391		6	max	268.562	3	5.021	9	6.162	3	0	3	.014	4	.396	2
392			min	-47.848	10	-87.869	2	-19.384	4	0	4	-.012	3	-.448	3
393		7	max	268.666	3	4.82	9	6.162	3	0	3	.009	4	.415	2
394			min	-47.732	10	-88.111	2	-19.142	4	0	4	-.011	3	-.437	3
395		8	max	268.771	3	4.618	9	6.162	3	0	3	.005	4	.434	2
396			min	-47.615	10	-88.353	2	-18.9	4	0	4	-.01	3	-.426	3
397		9	max	268.876	3	4.416	9	6.162	3	0	3	.001	4	.453	2
398			min	-47.499	10	-88.595	2	-18.658	4	0	4	-.008	3	-.415	3
399		10	max	268.98	3	4.215	9	6.162	3	0	3	0	10	.472	2
400			min	-47.383	10	-88.837	2	-18.416	4	0	4	-.007	3	-.404	3
401		11	max	269.085	3	4.013	9	6.162	3	0	3	0	10	.492	2
402			min	-47.266	10	-89.078	2	-18.174	4	0	4	-.007	4	-.393	3
403		12	max	269.19	3	3.812	9	6.162	3	0	3	0	10	.511	2
404			min	-47.15	10	-89.32	2	-17.932	4	0	4	-.011	4	-.381	3
405		13	max	269.295	3	3.61	9	6.162	3	0	3	0	10	.53	2
406			min	-47.034	10	-89.562	2	-17.69	4	0	4	-.015	4	-.37	3
407		14	max	269.399	3	3.409	9	6.162	3	0	3	0	10	.55	2
408			min	-46.917	10	-89.804	2	-17.448	4	0	4	-.018	4	-.359	3
409		15	max	269.504	3	3.207	9	6.162	3	0	3	0	10	.569	2
410			min	-46.801	10	-90.046	2	-17.206	4	0	4	-.022	4	-.348	3
411		16	max	300.635	2	437.178	2	6.133	3	0	3	0	3	.584	2
412			min	-23.194	3	-499.379	3	-15.895	4	0	4	-.026	4	-.332	3
413		17	max	300.775	2	436.936	2	6.133	3	0	3	.002	3	.49	2
414			min	-23.089	3	-499.56	3	-15.653	4	0	4	-.029	4	-.224	3
415		18	max	-8.633	12	1136.165	2	5.615	3	0	4	.003	3	.246	2
416			min	-259.468	1	-517.52	3	-38.527	5	0	1	-.038	4	-.112	3
417		19	max	-8.563	12	1135.923	2	5.615	3	0	4	.004	3	0	3
418			min	-259.328	1	-517.701	3	-38.285	5	0	1	-.046	4	0	2
419	M9	1	max	114.313	1	341.361	3	162.382	4	0	3	-.001	15	0	2
420			min	2.659	15	-248.508	1	3.719	10	0	1	-.089	1	0	3
421		2	max	114.453	1	341.18	3	162.624	4	0	3	.031	5	.054	1
422			min	2.702	15	-248.75	1	3.719	10	0	1	-.079	1	-.074	3
423		3	max	88.051	3	5.757	9	44.652	1	0	1	.062	5	.107	1
424			min	-11.991	10	-23.799	2	-25.708	5	0	5	-.068	1	-.147	3
425		4	max	88.156	3	5.556	9	44.652	1	0	1	.057	5	.109	2
426			min	-11.874	10	-24.04	2	-25.466	5	0	5	-.058	1	-.144	3
427		5	max	88.26	3	5.354	9	44.652	1	0	1	.051	5	.114	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-11.758	10	-24.282	2	-25.224	5	0	5	-.048	1	-.141	3
429	6	max	88.365	3	5.152	9	44.652	1	0	1	.046	5	.12	2
430		min	-11.642	10	-24.524	2	-24.982	5	0	5	-.039	1	-.138	3
431	7	max	88.47	3	4.951	9	44.652	1	0	1	.04	5	.125	2
432		min	-11.525	10	-24.766	2	-24.74	5	0	5	-.029	1	-.135	3
433	8	max	88.574	3	4.749	9	44.652	1	0	1	.035	5	.13	2
434		min	-11.409	10	-25.008	2	-24.498	5	0	5	-.019	1	-.132	3
435	9	max	88.679	3	4.548	9	44.652	1	0	1	.03	5	.136	2
436		min	-11.292	10	-25.249	2	-24.256	5	0	5	-.01	1	-.129	3
437	10	max	88.784	3	4.346	9	44.652	1	0	1	.024	4	.141	2
438		min	-11.176	10	-25.491	2	-24.014	5	0	5	0	1	-.126	3
439	11	max	88.889	3	4.145	9	44.652	1	0	1	.021	4	.147	2
440		min	-11.06	10	-25.733	2	-23.772	5	0	5	0	10	-.122	3
441	12	max	88.993	3	3.943	9	44.652	1	0	1	.019	1	.152	2
442		min	-10.943	10	-25.975	2	-23.53	5	0	5	.002	10	-.119	3
443	13	max	89.098	3	3.742	9	44.652	1	0	1	.029	1	.158	2
444		min	-10.827	10	-26.217	2	-23.288	5	0	5	.002	10	-.116	3
445	14	max	89.203	3	3.54	9	44.652	1	0	1	.039	1	.164	2
446		min	-10.711	10	-26.459	2	-23.046	5	0	5	.003	15	-.113	3
447	15	max	89.307	3	3.339	9	44.652	1	0	1	.048	1	.17	2
448		min	-10.594	10	-26.7	2	-22.804	5	0	5	0	5	-.109	3
449	16	max	91.861	2	111.233	2	44.988	1	0	10	.059	1	.174	2
450		min	-6.263	3	-162.888	3	-21.388	5	0	4	-.005	5	-.105	3
451	17	max	92.001	2	110.991	2	44.988	1	0	10	.068	1	.15	2
452		min	-6.159	3	-163.07	3	-21.146	5	0	4	-.009	5	-.069	3
453	18	max	2.874	5	347.504	2	47.333	1	0	2	.079	1	.076	2
454		min	-114.445	1	-159.214	3	-42.65	5	0	3	-.018	5	-.035	3
455	19	max	2.939	5	347.262	2	47.333	1	0	2	.089	1	0	2
456		min	-114.305	1	-159.396	3	-42.408	5	0	3	-.028	5	0	3
457	M13	1	max	162.387	4	248.184	1	-2.659	15	0	.089	1	0	1
458		min	3.721	10	-341.375	3	-114.303	1	0	3	.001	15	0	3
459	2	max	156.112	4	175.456	1	-1.567	15	0	2	.025	1	.186	3
460		min	3.721	10	-241.182	3	-87.172	1	0	3	0	5	-.135	1
461	3	max	149.837	4	102.729	1	-.475	15	0	2	.006	3	.308	3
462		min	3.721	10	-140.99	3	-60.042	1	0	3	-.022	1	-.224	1
463	4	max	143.563	4	30.001	1	.84	5	0	2	.002	3	.366	3
464		min	3.721	10	-40.798	3	-32.911	1	0	3	-.052	1	-.267	1
465	5	max	137.288	4	59.395	3	2.529	5	0	2	0	3	.36	3
466		min	3.721	10	-42.727	1	-5.78	1	0	3	-.065	1	-.263	1
467	6	max	131.013	4	159.587	3	21.35	1	0	2	.002	5	.29	3
468		min	3.721	10	-115.454	1	-1.077	3	0	3	-.06	1	-.212	1
469	7	max	124.738	4	259.779	3	48.481	1	0	2	.005	5	.156	3
470		min	3.721	10	-188.182	1	.436	12	0	3	-.037	1	-.115	1
471	8	max	118.463	4	359.972	3	75.612	1	0	2	.01	4	.028	1
472		min	3.721	10	-260.91	1	1.496	12	0	3	0	3	-.042	3
473	9	max	112.188	4	460.164	3	102.742	1	0	2	.059	1	.218	1
474		min	3.721	10	-333.637	1	2.555	12	0	3	.001	12	-.304	3
475	10	max	105.913	4	560.356	3	129.873	1	0	2	.134	1	.455	1
476		min	3.721	10	-406.365	1	3.615	12	0	3	.003	12	-.63	3
477	11	max	76.363	4	333.637	1	1.645	5	0	3	.058	1	.218	1
478		min	3.145	12	-460.164	3	-102.32	1	0	2	-.014	5	-.304	3
479	12	max	70.088	4	260.909	1	3.334	5	0	3	.003	2	.028	1
480		min	3.145	12	-359.972	3	-75.189	1	0	2	-.013	4	-.042	3
481	13	max	63.813	4	188.182	1	5.024	5	0	3	-.003	10	.156	3
482		min	3.145	12	-259.779	3	-48.059	1	0	2	-.038	1	-.115	1
483	14	max	57.538	4	115.454	1	6.713	5	0	3	-.004	15	.29	3
484		min	3.145	12	-159.587	3	-20.928	1	0	2	-.06	1	-.212	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	51.263	4	42.726	1	10.027	4	0	3	0	15	.36	3
486			min	3.145	12	-59.395	3	-.317	10	0	2	-.064	1	-.263	1
487		16	max	46.012	1	40.798	3	33.333	1	0	3	.005	5	.366	3
488			min	3.145	12	-30.001	1	2.7	10	0	2	-.052	1	-.267	1
489		17	max	46.012	1	140.99	3	60.464	1	0	3	.012	5	.308	3
490			min	3.145	12	-102.729	1	4.079	12	0	2	-.022	1	-.224	1
491		18	max	46.012	1	241.183	3	87.594	1	0	3	.026	4	.186	3
492			min	3.145	12	-175.457	1	5.139	12	0	2	0	10	-.135	1
493		19	max	46.012	1	341.375	3	114.725	1	0	3	.09	1	0	1
494			min	3.145	12	-248.184	1	6.198	12	0	2	.007	12	0	3
495	M16	1	max	42.402	5	347.439	2	2.939	5	0	3	.089	1	0	2
496			min	-47.202	1	-159.421	3	-114.316	1	0	2	-.028	5	0	3
497		2	max	36.127	5	245.64	2	4.628	5	0	3	.025	1	.087	3
498			min	-47.202	1	-112.94	3	-87.186	1	0	2	-.025	5	-.189	2
499		3	max	29.852	5	143.842	2	6.317	5	0	3	0	12	.144	3
500			min	-47.202	1	-66.458	3	-60.055	1	0	2	-.026	4	-.314	2
501		4	max	23.577	5	42.043	2	8.007	5	0	3	-.002	12	.172	3
502			min	-47.202	1	-19.976	3	-32.924	1	0	2	-.052	1	-.373	2
503		5	max	17.302	5	26.506	3	9.696	5	0	3	-.003	12	.17	3
504			min	-47.202	1	-59.756	2	-5.794	1	0	2	-.065	1	-.368	2
505		6	max	11.027	5	72.987	3	21.337	1	0	3	-.003	15	.138	3
506			min	-47.202	1	-161.554	2	-.326	3	0	2	-.06	1	-.297	2
507		7	max	4.752	5	119.469	3	48.468	1	0	3	.003	5	.077	3
508			min	-47.202	1	-263.353	2	.906	12	0	2	-.037	1	-.161	2
509		8	max	.358	3	165.951	3	75.598	1	0	3	.012	4	.04	2
510			min	-47.202	1	-365.152	2	1.965	12	0	2	-.004	3	-.015	3
511		9	max	.358	3	212.433	3	102.729	1	0	3	.059	1	.305	2
512			min	-47.202	1	-466.95	2	3.025	12	0	2	-.002	3	-.135	3
513		10	max	24.232	5	-10.564	15	129.86	1	0	14	.134	1	.636	2
514			min	-47.202	1	-568.749	2	-6.666	3	0	2	.004	12	-.286	3
515		11	max	17.957	5	466.95	2	1.374	5	0	2	.059	1	.305	2
516			min	-47.034	1	-212.433	3	-102.317	1	0	3	-.012	5	-.135	3
517		12	max	11.682	5	365.152	2	3.063	5	0	2	.003	2	.04	2
518			min	-47.034	1	-165.951	3	-75.186	1	0	3	-.01	4	-.015	3
519		13	max	5.407	5	263.353	2	4.752	5	0	2	-.002	12	.077	3
520			min	-47.034	1	-119.469	3	-48.056	1	0	3	-.038	1	-.161	2
521		14	max	-.507	15	161.554	2	6.442	5	0	2	-.002	12	.138	3
522			min	-47.034	1	-72.987	3	-20.925	1	0	3	-.06	1	-.297	2
523		15	max	-3.408	12	59.756	2	9.733	4	0	2	0	15	.17	3
524			min	-47.034	1	-26.506	3	-.305	10	0	3	-.064	1	-.368	2
525		16	max	-3.408	12	19.976	3	33.336	1	0	2	.006	5	.172	3
526			min	-47.034	1	-42.043	2	1.878	12	0	3	-.052	1	-.373	2
527		17	max	-3.408	12	66.458	3	60.467	1	0	2	.013	5	.144	3
528			min	-47.034	1	-143.842	2	2.937	12	0	3	-.022	1	-.314	2
529		18	max	-3.408	12	112.94	3	87.598	1	0	2	.027	4	.087	3
530			min	-47.034	1	-245.64	2	3.997	12	0	3	0	10	-.189	2
531		19	max	-3.408	12	159.421	3	114.728	1	0	2	.09	1	0	2
532			min	-47.034	1	-347.439	2	5.056	12	0	3	.006	12	0	3
533	M15	1	max	0	1	1.313	9	.068	3	0	9	0	9	0	1
534			min	-69.539	3	0	1	-.021	9	0	3	0	3	0	1
535		2	max	0	1	1.167	9	.068	3	0	9	0	9	0	1
536			min	-69.61	3	0	1	-.021	9	0	3	0	3	0	9
537		3	max	0	1	1.021	9	.068	3	0	9	0	9	0	1
538			min	-69.68	3	0	1	-.021	9	0	3	0	3	0	9
539		4	max	0	1	.875	9	.068	3	0	9	0	9	0	1
540			min	-69.751	3	0	1	-.021	9	0	3	0	3	-.001	9
541		5	max	0	1	.729	9	.068	3	0	9	0	9	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-69.821	3	0	1	-.021	9	0	3	0	3	-.002	9
543		6	max	0	1	.583	9	.068	3	0	9	0	9	0	1
544			min	-69.892	3	0	1	-.021	9	0	3	0	3	-.002	9
545		7	max	0	1	.438	9	.068	3	0	9	0	3	0	1
546			min	-69.962	3	0	1	-.021	9	0	3	0	9	-.002	9
547		8	max	0	1	.292	9	.068	3	0	9	0	3	0	1
548			min	-70.033	3	0	1	-.021	9	0	3	0	9	-.002	9
549		9	max	0	1	.146	9	.068	3	0	9	0	3	0	1
550			min	-70.103	3	0	1	-.021	9	0	3	0	9	-.002	9
551		10	max	0	1	0	1	.068	3	0	9	0	3	0	1
552			min	-70.174	3	0	1	-.021	9	0	3	0	9	-.002	9
553		11	max	0	1	0	1	.068	3	0	9	0	3	0	1
554			min	-70.244	3	-.146	9	-.021	9	0	3	0	9	-.002	9
555		12	max	0	1	0	1	.068	3	0	9	0	3	0	1
556			min	-70.315	3	-.292	9	-.021	9	0	3	0	9	-.002	9
557		13	max	0	1	0	1	.068	3	0	9	0	3	0	1
558			min	-70.385	3	-.438	9	-.021	9	0	3	0	9	-.002	9
559		14	max	0	1	0	1	.068	3	0	9	0	3	0	1
560			min	-70.456	3	-.583	9	-.021	9	0	3	0	9	-.002	9
561		15	max	0	1	0	1	.068	3	0	9	0	3	0	1
562			min	-70.526	3	-.729	9	-.021	9	0	3	0	9	-.002	9
563		16	max	0	1	0	1	.068	3	0	9	0	3	0	1
564			min	-70.597	3	-.875	9	-.021	9	0	3	0	9	-.001	9
565		17	max	0	1	0	1	.068	3	0	9	0	3	0	1
566			min	-70.667	3	-1.021	9	-.021	9	0	3	0	9	0	9
567		18	max	0	1	0	1	.068	3	0	9	0	3	0	1
568			min	-70.738	3	-1.167	9	-.021	9	0	3	0	9	0	9
569		19	max	0	1	0	1	.068	3	0	9	0	3	0	1
570			min	-70.808	3	-1.313	9	-.021	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.744	4	.295	4	0	3	0	3	0	1
572			min	-208.932	4	0	10	-.028	3	0	2	0	4	0	1
573		2	max	0	10	2.439	4	.265	4	0	3	0	3	0	10
574			min	-208.961	4	0	10	-.028	3	0	2	0	4	0	4
575		3	max	0	10	2.134	4	.236	4	0	3	0	3	0	10
576			min	-208.99	4	0	10	-.028	3	0	2	0	4	-.002	4
577		4	max	0	10	1.829	4	.207	4	0	3	0	3	0	10
578			min	-209.019	4	0	10	-.028	3	0	2	0	4	-.003	4
579		5	max	0	10	1.525	4	.178	4	0	3	0	3	0	10
580			min	-209.047	4	0	10	-.028	3	0	2	0	1	-.003	4
581		6	max	0	10	1.22	4	.148	4	0	3	0	3	0	10
582			min	-209.076	4	0	10	-.028	3	0	2	0	1	-.004	4
583		7	max	0	10	.915	4	.119	4	0	3	0	5	0	10
584			min	-209.105	4	0	10	-.028	3	0	2	0	1	-.004	4
585		8	max	0	10	.61	4	.09	4	0	3	0	5	0	10
586			min	-209.134	4	0	10	-.028	3	0	2	0	1	-.004	4
587		9	max	0	10	.305	4	.061	4	0	3	0	5	0	10
588			min	-209.163	4	0	10	-.028	3	0	2	0	1	-.004	4
589		10	max	0	10	0	1	.031	4	0	3	0	5	0	10
590			min	-209.192	4	0	1	-.028	3	0	2	0	1	-.005	4
591		11	max	0	10	0	10	.028	1	0	3	0	5	0	10
592			min	-209.221	4	-.305	4	-.028	3	0	2	0	1	-.004	4
593		12	max	0	10	0	10	.028	1	0	3	0	5	0	10
594			min	-209.25	4	-.61	4	-.031	5	0	2	0	1	-.004	4
595		13	max	0	10	0	10	.028	1	0	3	0	5	0	10
596			min	-209.278	4	-.915	4	-.06	5	0	2	0	3	-.004	4
597		14	max	0	10	0	10	.028	1	0	3	0	5	0	10
598			min	-209.307	4	-1.22	4	-.089	5	0	2	0	3	-.004	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.088	2	0	10	.028	1	0	3	0	4	0	10
600		min	-209.336	4	-1.525	4	-.119	5	0	2	0	3	-.003	4
601	16	max	.182	2	0	10	.028	1	0	3	0	4	0	10
602		min	-209.365	4	-1.829	4	-.148	5	0	2	0	3	-.003	4
603	17	max	.276	2	0	10	.028	1	0	3	0	1	0	10
604		min	-209.394	4	-2.134	4	-.177	5	0	2	0	3	-.002	4
605	18	max	.37	2	0	10	.028	1	0	3	0	1	0	10
606		min	-209.423	4	-2.439	4	-.206	5	0	2	0	5	0	4
607	19	max	.464	2	0	10	.028	1	0	3	0	1	0	1
608		min	-209.452	4	-2.744	4	-.236	5	0	2	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.009	2	.009	1	1.388e-3	5	NC	3	NC	2
2			min	-.004	3	-.009	3	-.013	5	-7.22e-4	1	4245.779	2	4239.015	1
3		2	max	.002	1	.009	2	.009	1	1.409e-3	5	NC	3	NC	2
4			min	-.003	3	-.009	3	-.013	5	-6.911e-4	1	4634.521	2	4567.728	1
5		3	max	.002	1	.008	2	.008	1	1.431e-3	5	NC	3	NC	2
6			min	-.003	3	-.009	3	-.013	5	-6.603e-4	1	5096.998	2	4956.086	1
7		4	max	.002	1	.007	2	.007	1	1.453e-3	5	NC	1	NC	2
8			min	-.003	3	-.008	3	-.012	5	-6.294e-4	1	5651.025	2	5418.371	1
9		5	max	.002	1	.006	2	.007	1	1.474e-3	5	NC	1	NC	2
10			min	-.003	3	-.008	3	-.012	5	-5.985e-4	1	6320.356	2	5973.656	1
11		6	max	.002	1	.006	2	.006	1	1.496e-3	5	NC	1	NC	2
12			min	-.003	3	-.007	3	-.012	5	-5.677e-4	1	7137.176	2	6647.87	1
13		7	max	.002	1	.005	2	.005	1	1.517e-3	5	NC	1	NC	2
14			min	-.002	3	-.007	3	-.011	5	-5.368e-4	1	8145.933	2	7477.002	1
15		8	max	.001	1	.004	2	.005	1	1.539e-3	5	NC	1	NC	2
16			min	-.002	3	-.007	3	-.01	5	-5.06e-4	1	9409.379	2	8512.192	1
17		9	max	.001	1	.004	2	.004	1	1.561e-3	5	NC	1	NC	2
18			min	-.002	3	-.006	3	-.01	5	-4.751e-4	1	NC	1	9828.174	1
19		10	max	.001	1	.003	2	.003	1	1.582e-3	5	NC	1	NC	1
20		min	-.002	3	-.006	3	-.009	5	-4.442e-4	1	NC	1	NC	1	
21	11	max	.001	1	.002	2	.003	1	1.604e-3	5	NC	1	NC	1	
22		min	-.002	3	-.005	3	-.008	5	-4.134e-4	1	NC	1	NC	1	
23	12	max	0	1	.002	2	.002	1	1.625e-3	5	NC	1	NC	1	
24		min	-.001	3	-.005	3	-.008	5	-3.825e-4	1	NC	1	NC	1	
25	13	max	0	1	.002	2	.002	1	1.647e-3	5	NC	1	NC	1	
26		min	-.001	3	-.004	3	-.007	5	-3.516e-4	1	NC	1	NC	1	
27	14	max	0	1	.001	2	.001	1	1.669e-3	5	NC	1	NC	1	
28		min	0	3	-.003	3	-.006	5	-3.208e-4	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	1	1.69e-3	5	NC	1	NC	1	
30		min	0	3	-.003	3	-.005	5	-2.899e-4	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.712e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.004	5	-2.59e-4	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.734e-3	5	NC	1	NC	1	
34		min	0	3	-.001	3	-.002	5	-2.282e-4	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.755e-3	5	NC	1	NC	1	
36		min	0	3	0	3	-.001	5	-1.973e-4	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.777e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-1.665e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	7.85e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-8.376e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.004	5	9.588e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-8.469e-4	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.009	5	1.133e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-8.561e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.013	5	1.306e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-8.654e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.017	5	1.48e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-8.746e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.021	5	1.654e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-8.838e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.026	4	1.828e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-8.931e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.03	4	2.002e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-9.023e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.034	4	2.175e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-9.115e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.038	4	2.349e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-9.208e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.042	4	2.523e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-9.3e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.046	4	2.697e-4	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	10	-9.393e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.05	4	2.87e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-9.485e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.054	4	3.044e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	10	-9.577e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.057	4	3.218e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	-9.67e-4	5	9079.889	2	NC	1
69		16	max	.002	3	.006	2	.061	4	3.392e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-9.762e-4	5	7681.422	2	NC	1
71		17	max	.002	3	.007	2	.065	4	3.566e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-9.854e-4	5	6601.978	2	NC	1
73		18	max	.002	3	.008	2	.068	4	3.739e-4	1	NC	3	NC	1
74			min	-.002	2	-.008	3	0	10	-9.947e-4	5	5758.991	2	NC	1
75		19	max	.002	3	.009	2	.072	4	3.913e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-1.004e-3	5	5094.604	2	NC	1
77	M4	1	max	.002	1	.011	2	0	10	5.001e-3	5	NC	1	NC	2
78			min	0	12	-.009	3	-.076	4	-5.817e-4	1	NC	1	255.453	4
79		2	max	.002	1	.01	2	0	10	5.001e-3	5	NC	1	NC	2
80			min	0	12	-.009	3	-.069	4	-5.817e-4	1	NC	1	278.469	4
81		3	max	.002	1	.01	2	0	10	5.001e-3	5	NC	1	NC	2
82			min	0	12	-.008	3	-.063	4	-5.817e-4	1	NC	1	305.861	4
83		4	max	.001	1	.009	2	0	10	5.001e-3	5	NC	1	NC	2
84			min	0	12	-.008	3	-.057	4	-5.817e-4	1	NC	1	338.784	4
85		5	max	.001	1	.008	2	0	10	5.001e-3	5	NC	1	NC	2
86			min	0	12	-.007	3	-.051	4	-5.817e-4	1	NC	1	378.808	4
87		6	max	.001	1	.008	2	0	10	5.001e-3	5	NC	1	NC	2
88			min	0	12	-.007	3	-.045	4	-5.817e-4	1	NC	1	428.117	4
89		7	max	.001	1	.007	2	0	10	5.001e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.039	4	-5.817e-4	1	NC	1	489.821	4
91		8	max	.001	1	.007	2	0	10	5.001e-3	5	NC	1	NC	1
92			min	0	12	-.006	3	-.034	4	-5.817e-4	1	NC	1	568.475	4
93		9	max	0	1	.006	2	0	10	5.001e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.029	4	-5.817e-4	1	NC	1	670.968	4
95		10	max	0	1	.005	2	0	10	5.001e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.024	4	-5.817e-4	1	NC	1	808.129	4
97		11	max	0	1	.005	2	0	10	5.001e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.019	4	-5.817e-4	1	NC	1	997.793	4
99		12	max	0	1	.004	2	0	10	5.001e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.004	3	-.015	4	-5.817e-4	1	NC	1	1271.068	4
101		max	0	1	.004	2	0	10	5.001e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.011	4	-5.817e-4	1	NC	1	1686.306	4
103		max	0	1	.003	2	0	10	5.001e-3	5	NC	1	NC	1
104		min	0	12	-.003	3	-.008	4	-5.817e-4	1	NC	1	2363.868	4
105		max	0	1	.002	2	0	10	5.001e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.005	4	-5.817e-4	1	NC	1	3586.515	4
107		max	0	1	.002	2	0	10	5.001e-3	5	NC	1	NC	1
108		min	0	12	-.002	3	-.003	4	-5.817e-4	1	NC	1	6158.068	4
109		max	0	1	.001	2	0	10	5.001e-3	5	NC	1	NC	1
110		min	0	12	-.001	3	-.001	4	-5.817e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	5.001e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-5.817e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	5.001e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-5.817e-4	1	NC	1	NC	1
115	M6	max	.008	1	.032	2	.004	1	1.504e-3	4	NC	3	NC	1
116		min	-.011	3	-.029	3	-.013	5	-6.079e-8	10	1214.524	2	8290.745	3
117		max	.007	1	.03	2	.004	1	1.524e-3	4	NC	3	NC	1
118		min	-.011	3	-.028	3	-.013	5	-5.739e-8	10	1299.26	2	8811.458	3
119		max	.007	1	.028	2	.003	1	1.543e-3	4	NC	3	NC	1
120		min	-.01	3	-.026	3	-.013	5	-1.563e-6	2	1396.313	2	9428.284	3
121		max	.006	1	.026	2	.003	1	1.563e-3	4	NC	3	NC	1
122		min	-.009	3	-.025	3	-.013	5	-3.33e-6	1	1508.147	2	NC	1
123		max	.006	1	.024	2	.003	1	1.583e-3	4	NC	3	NC	1
124		min	-.009	3	-.023	3	-.012	5	-7.915e-6	1	1637.933	2	NC	1
125		max	.006	1	.022	2	.002	1	1.603e-3	4	NC	3	NC	1
126		min	-.008	3	-.022	3	-.012	5	-1.25e-5	1	1789.825	2	NC	1
127		max	.005	1	.02	2	.002	1	1.622e-3	4	NC	3	NC	1
128		min	-.008	3	-.02	3	-.011	5	-1.708e-5	1	1969.364	2	NC	1
129		max	.005	1	.018	2	.002	1	1.642e-3	4	NC	3	NC	1
130		min	-.007	3	-.018	3	-.011	5	-2.167e-5	1	2184.109	2	NC	1
131		max	.004	1	.016	2	.002	1	1.662e-3	4	NC	3	NC	1
132		min	-.006	3	-.017	3	-.01	5	-2.625e-5	1	2444.64	2	NC	1
133		max	.004	1	.014	2	.001	1	1.681e-3	4	NC	3	NC	1
134		min	-.006	3	-.015	3	-.009	5	-3.084e-5	1	2766.243	2	NC	1
135		max	.003	1	.012	2	.001	1	1.701e-3	4	NC	3	NC	1
136		min	-.005	3	-.014	3	-.009	5	-3.542e-5	1	3171.835	2	NC	1
137		max	.003	1	.011	2	0	1	1.721e-3	4	NC	3	NC	1
138		min	-.004	3	-.012	3	-.008	5	-4.001e-5	1	3697.424	2	NC	1
139		max	.003	1	.009	2	0	1	1.74e-3	4	NC	3	NC	1
140		min	-.004	3	-.01	3	-.007	5	-4.459e-5	1	4403.002	2	NC	1
141		max	.002	1	.007	2	0	1	1.76e-3	4	NC	3	NC	1
142		min	-.003	3	-.009	3	-.006	5	-4.918e-5	1	5396.528	2	NC	1
143		max	.002	1	.006	2	0	1	1.78e-3	4	NC	1	NC	1
144		min	-.003	3	-.007	3	-.005	5	-5.376e-5	1	6893.875	2	NC	1
145		max	.001	1	.004	2	0	1	1.8e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.004	5	-5.834e-5	1	9398.664	2	NC	1
147		max	0	1	.003	2	0	1	1.819e-3	4	NC	1	NC	1
148		min	-.001	3	-.003	3	-.003	5	-6.293e-5	1	NC	1	NC	1
149		max	0	1	.001	2	0	1	1.84e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-6.751e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.86e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-7.21e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	3.37e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-8.769e-4	5	NC	1	NC	1
155		max	0	3	.001	2	.004	5	2.826e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-8.726e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.009	5	2.282e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-8.691e-4	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.013	5	1.739e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-8.656e-4	4	NC	1	NC	1
161		5	max	.001	3	.006	2	.018	5	1.195e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-8.621e-4	4	8058.164	2	NC	1
163		6	max	.002	3	.007	2	.022	5	2.269e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	-8.585e-4	4	6459.907	2	NC	1
165		7	max	.002	3	.009	2	.027	5	4.266e-5	3	NC	3	NC	1
166			min	-.002	2	-.011	3	0	1	-8.55e-4	4	5368.754	2	NC	1
167		8	max	.002	3	.01	2	.031	4	6.263e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	-.001	1	-8.515e-4	4	4570.094	2	NC	1
169		9	max	.003	3	.012	2	.035	4	8.26e-5	3	NC	3	NC	1
170			min	-.003	2	-.015	3	-.001	1	-8.48e-4	4	3956.964	2	NC	1
171		10	max	.003	3	.013	2	.039	4	1.026e-4	3	NC	3	NC	1
172			min	-.004	2	-.016	3	-.001	1	-8.444e-4	4	3470.066	2	NC	1
173		11	max	.003	3	.015	2	.043	4	1.225e-4	3	NC	3	NC	1
174			min	-.004	2	-.018	3	-.001	1	-8.409e-4	4	3073.808	2	NC	1
175		12	max	.004	3	.017	2	.047	4	1.425e-4	3	NC	3	NC	1
176			min	-.004	2	-.019	3	-.002	1	-8.374e-4	4	2745.435	2	NC	1
177		13	max	.004	3	.019	2	.051	4	1.625e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	-.002	1	-8.339e-4	4	2469.623	2	NC	1
179		14	max	.004	3	.021	2	.055	4	1.824e-4	3	NC	3	NC	1
180			min	-.005	2	-.021	3	-.002	1	-8.303e-4	4	2235.597	2	NC	1
181		15	max	.005	3	.023	2	.058	4	2.024e-4	3	NC	3	NC	1
182			min	-.006	2	-.022	3	-.002	1	-8.268e-4	4	2035.497	2	NC	1
183		16	max	.005	3	.025	2	.062	4	2.224e-4	3	NC	3	NC	1
184			min	-.006	2	-.023	3	-.002	1	-8.233e-4	4	1863.413	2	NC	1
185		17	max	.005	3	.027	2	.066	4	2.424e-4	3	NC	3	NC	1
186			min	-.006	2	-.024	3	-.002	1	-8.198e-4	4	1714.789	2	NC	1
187		18	max	.006	3	.029	2	.069	4	2.623e-4	3	NC	3	NC	1
188			min	-.007	2	-.025	3	-.002	1	-8.162e-4	4	1586.043	2	NC	1
189		19	max	.006	3	.031	2	.073	4	2.823e-4	3	NC	3	NC	1
190			min	-.007	2	-.026	3	-.002	1	-8.127e-4	4	1474.317	2	NC	1
191	M8	1	max	.005	1	.037	2	.002	1	4.817e-3	4	NC	1	NC	2
192			min	0	3	-.029	3	-.076	4	-2.179e-4	3	NC	1	253.494	4
193		2	max	.004	1	.035	2	.002	1	4.817e-3	4	NC	1	NC	1
194			min	0	3	-.028	3	-.07	4	-2.179e-4	3	NC	1	276.333	4
195		3	max	.004	1	.033	2	.002	1	4.817e-3	4	NC	1	NC	1
196			min	0	3	-.026	3	-.064	4	-2.179e-4	3	NC	1	303.515	4
197		4	max	.004	1	.031	2	.002	1	4.817e-3	4	NC	1	NC	1
198			min	0	3	-.024	3	-.057	4	-2.179e-4	3	NC	1	336.185	4
199		5	max	.004	1	.029	2	.001	1	4.817e-3	4	NC	1	NC	1
200			min	0	3	-.023	3	-.051	4	-2.179e-4	3	NC	1	375.901	4
201		6	max	.003	1	.027	2	.001	1	4.817e-3	4	NC	1	NC	1
202			min	0	3	-.021	3	-.045	4	-2.179e-4	3	NC	1	424.831	4
203		7	max	.003	1	.025	2	.001	1	4.817e-3	4	NC	1	NC	1
204			min	0	3	-.019	3	-.04	4	-2.179e-4	3	NC	1	486.061	4
205		8	max	.003	1	.023	2	0	1	4.817e-3	4	NC	1	NC	1
206			min	0	3	-.018	3	-.034	4	-2.179e-4	3	NC	1	564.111	4
207		9	max	.003	1	.021	2	0	1	4.817e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.029	4	-2.179e-4	3	NC	1	665.817	4
209		10	max	.002	1	.019	2	0	1	4.817e-3	4	NC	1	NC	1
210			min	0	3	-.015	3	-.024	4	-2.179e-4	3	NC	1	801.925	4
211		11	max	.002	1	.016	2	0	1	4.817e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.02	4	-2.179e-4	3	NC	1	990.133	4
213		12	max	.002	1	.014	2	0	1	4.817e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214		min	0	3	-.011	3	-.015	4	-2.179e-4	3	NC	1	1261.311	4
215		max	.002	1	.012	2	0	1	4.817e-3	4	NC	1	NC	1
216		min	0	3	-.01	3	-.012	4	-2.179e-4	3	NC	1	1673.363	4
217		max	.001	1	.01	2	0	1	4.817e-3	4	NC	1	NC	1
218		min	0	3	-.008	3	-.008	4	-2.179e-4	3	NC	1	2345.725	4
219		max	.001	1	.008	2	0	1	4.817e-3	4	NC	1	NC	1
220		min	0	3	-.006	3	-.005	4	-2.179e-4	3	NC	1	3558.99	4
221		max	0	1	.006	2	0	1	4.817e-3	4	NC	1	NC	1
222		min	0	3	-.005	3	-.003	4	-2.179e-4	3	NC	1	6110.811	4
223		max	0	1	.004	2	0	1	4.817e-3	4	NC	1	NC	1
224		min	0	3	-.003	3	-.001	4	-2.179e-4	3	NC	1	NC	1
225		max	0	1	.002	2	0	1	4.817e-3	4	NC	1	NC	1
226		min	0	3	-.002	3	0	4	-2.179e-4	3	NC	1	NC	1
227		max	0	1	0	1	0	1	4.817e-3	4	NC	1	NC	1
228		min	0	1	0	1	0	1	-2.179e-4	3	NC	1	NC	1
229	M10	max	.002	1	.009	2	0	3	7.343e-4	1	NC	3	NC	1
230		min	-.003	3	-.009	3	-.006	4	-4.218e-4	3	4250.127	2	NC	1
231		max	.002	1	.008	2	0	3	6.963e-4	1	NC	3	NC	1
232		min	-.003	3	-.009	3	-.006	4	-4.077e-4	3	4639.412	2	NC	1
233		max	.002	1	.008	2	0	3	6.584e-4	1	NC	3	NC	1
234		min	-.003	3	-.009	3	-.006	4	-3.936e-4	3	5102.562	2	NC	1
235		max	.002	1	.007	2	0	3	6.204e-4	1	NC	1	NC	1
236		min	-.003	3	-.008	3	-.006	4	-3.795e-4	3	5657.434	2	NC	1
237		max	.002	1	.006	2	0	3	6.163e-4	4	NC	1	NC	1
238		min	-.002	3	-.008	3	-.007	4	-3.654e-4	3	6327.834	2	NC	1
239		max	.002	1	.006	2	0	3	6.767e-4	4	NC	1	NC	1
240		min	-.002	3	-.007	3	-.007	4	-3.512e-4	3	7146.025	2	NC	1
241		max	.002	1	.005	2	0	3	7.372e-4	4	NC	1	NC	1
242		min	-.002	3	-.007	3	-.007	4	-3.371e-4	3	8156.565	2	NC	1
243		max	.001	1	.004	2	0	3	7.977e-4	4	NC	1	NC	1
244		min	-.002	3	-.007	3	-.006	4	-3.23e-4	3	9422.373	2	NC	1
245		max	.001	1	.004	2	0	3	8.581e-4	4	NC	1	NC	1
246		min	-.002	3	-.006	3	-.006	4	-3.089e-4	3	NC	1	NC	1
247		max	.001	1	.003	2	0	3	9.186e-4	4	NC	1	NC	1
248		min	-.002	3	-.006	3	-.006	4	-2.948e-4	3	NC	1	NC	1
249		max	.001	1	.002	2	0	3	9.79e-4	4	NC	1	NC	1
250		min	-.001	3	-.005	3	-.006	4	-2.807e-4	3	NC	1	NC	1
251		max	0	1	.002	2	0	3	1.04e-3	4	NC	1	NC	1
252		min	-.001	3	-.005	3	-.005	4	-2.666e-4	3	NC	1	NC	1
253		max	0	1	.002	2	0	3	1.1e-3	4	NC	1	NC	1
254		min	-.001	3	-.004	3	-.005	4	-2.525e-4	3	NC	1	NC	1
255		max	0	1	.001	2	0	3	1.16e-3	4	NC	1	NC	1
256		min	0	3	-.003	3	-.004	4	-2.383e-4	3	NC	1	NC	1
257		max	0	1	0	2	0	3	1.221e-3	4	NC	1	NC	1
258		min	0	3	-.003	3	-.004	4	-2.242e-4	3	NC	1	NC	1
259		max	0	1	0	2	0	3	1.281e-3	4	NC	1	NC	1
260		min	0	3	-.002	3	-.003	4	-2.101e-4	3	NC	1	NC	1
261		max	0	1	0	2	0	3	1.342e-3	4	NC	1	NC	1
262		min	0	3	-.001	3	-.002	4	-1.96e-4	3	NC	1	NC	1
263		max	0	1	0	2	0	3	1.402e-3	4	NC	1	NC	1
264		min	0	3	0	3	0	4	-1.819e-4	3	NC	1	NC	1
265		max	0	1	0	1	0	1	1.463e-3	4	NC	1	NC	1
266		min	0	1	0	1	0	1	-1.678e-4	3	NC	1	NC	1
267	M11	max	0	1	0	1	0	1	7.916e-5	3	NC	1	NC	1
268		min	0	1	0	1	0	1	-6.903e-4	4	NC	1	NC	1
269		max	0	3	0	2	.004	4	5.881e-5	3	NC	1	NC	1
270		min	0	2	0	3	0	3	-7.632e-4	4	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.007	4	3.847e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-8.361e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.011	4	1.812e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-9.09e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.014	4	-1.84e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-9.819e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.018	5	-1.368e-5	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-1.055e-3	4	NC	1	NC	1
279		7	max	0	3	0	2	.022	5	-1.556e-5	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-1.128e-3	4	NC	1	NC	1
281		8	max	0	3	0	2	.025	5	-1.745e-5	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-1.201e-3	4	NC	1	NC	1
283		9	max	0	3	.001	2	.029	5	-1.933e-5	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-1.274e-3	4	NC	1	NC	1
285		10	max	0	3	.002	2	.032	5	-2.121e-5	10	NC	1	NC	1
286			min	0	2	-.007	3	-.003	1	-1.346e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.036	5	-2.31e-5	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	1	-1.419e-3	4	NC	1	NC	1
289		12	max	.001	3	.003	2	.039	5	-2.498e-5	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.004	1	-1.492e-3	4	NC	1	NC	1
291		13	max	.001	3	.003	2	.043	5	-2.686e-5	10	NC	1	NC	2
292			min	-.001	2	-.008	3	-.005	1	-1.565e-3	4	NC	1	9942.87	1
293		14	max	.001	3	.004	2	.046	5	-2.874e-5	10	NC	1	NC	2
294			min	-.001	2	-.008	3	-.005	1	-1.638e-3	4	NC	1	8644.378	1
295		15	max	.002	3	.005	2	.05	5	-3.063e-5	10	NC	1	NC	2
296			min	-.001	2	-.008	3	-.006	1	-1.711e-3	4	9094.829	2	7635.23	1
297		16	max	.002	3	.006	2	.053	5	-3.251e-5	10	NC	1	NC	2
298			min	-.002	2	-.008	3	-.007	1	-1.784e-3	4	7692.868	2	6837.092	1
299		17	max	.002	3	.007	2	.057	5	-3.439e-5	10	NC	1	NC	2
300			min	-.002	2	-.008	3	-.007	1	-1.857e-3	4	6610.989	2	6197.055	1
301		18	max	.002	3	.008	2	.06	5	-3.628e-5	10	NC	3	NC	2
302			min	-.002	2	-.008	3	-.008	1	-1.93e-3	4	5766.271	2	5678.367	1
303		19	max	.002	3	.009	2	.064	5	-3.816e-5	10	NC	3	NC	2
304			min	-.002	2	-.008	3	-.009	1	-2.003e-3	4	5100.632	2	5254.906	1
305	M12	1	max	.002	1	.011	2	.007	1	5.892e-3	4	NC	1	NC	3
306			min	0	12	-.009	3	-.07	5	4.337e-5	10	NC	1	276.442	5
307		2	max	.002	1	.01	2	.007	1	5.892e-3	4	NC	1	NC	3
308			min	0	12	-.009	3	-.064	5	4.337e-5	10	NC	1	301.341	5
309		3	max	.002	1	.01	2	.006	1	5.892e-3	4	NC	1	NC	3
310			min	0	12	-.008	3	-.058	5	4.337e-5	10	NC	1	330.976	5
311		4	max	.001	1	.009	2	.006	1	5.892e-3	4	NC	1	NC	2
312			min	0	12	-.008	3	-.053	5	4.337e-5	10	NC	1	366.593	5
313		5	max	.001	1	.008	2	.005	1	5.892e-3	4	NC	1	NC	2
314			min	0	12	-.007	3	-.047	5	4.337e-5	10	NC	1	409.891	5
315		6	max	.001	1	.008	2	.004	1	5.892e-3	4	NC	1	NC	2
316			min	0	12	-.007	3	-.042	5	4.337e-5	10	NC	1	463.233	5
317		7	max	.001	1	.007	2	.004	1	5.892e-3	4	NC	1	NC	2
318			min	0	12	-.006	3	-.036	5	4.337e-5	10	NC	1	529.984	5
319		8	max	.001	1	.007	2	.003	1	5.892e-3	4	NC	1	NC	2
320			min	0	12	-.006	3	-.031	5	4.337e-5	10	NC	1	615.069	5
321		9	max	0	1	.006	2	.003	1	5.892e-3	4	NC	1	NC	2
322			min	0	12	-.005	3	-.027	5	4.337e-5	10	NC	1	725.941	5
323		10	max	0	1	.005	2	.002	1	5.892e-3	4	NC	1	NC	2
324			min	0	12	-.005	3	-.022	5	4.337e-5	10	NC	1	874.313	5
325		11	max	0	1	.005	2	.002	1	5.892e-3	4	NC	1	NC	1
326			min	0	12	-.004	3	-.018	5	4.337e-5	10	NC	1	1079.476	5
327		12	max	0	1	.004	2	.001	1	5.892e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	12	-.004	3	-.014	5	4.337e-5	10	NC	1	1375.078	5
329		max	0	1	.004	2	.001	1	5.892e-3	4	NC	1	NC	1
330		min	0	12	-.003	3	-.011	5	4.337e-5	10	NC	1	1824.234	5
331		max	0	1	.003	2	0	1	5.892e-3	4	NC	1	NC	1
332		min	0	12	-.003	3	-.008	5	4.337e-5	10	NC	1	2557.129	5
333		max	0	1	.002	2	0	1	5.892e-3	4	NC	1	NC	1
334		min	0	12	-.002	3	-.005	5	4.337e-5	10	NC	1	3879.6	5
335		max	0	1	.002	2	0	1	5.892e-3	4	NC	1	NC	1
336		min	0	12	-.002	3	-.003	5	4.337e-5	10	NC	1	6661.059	5
337		max	0	1	.001	2	0	1	5.892e-3	4	NC	1	NC	1
338		min	0	12	-.001	3	-.001	5	4.337e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	5.892e-3	4	NC	1	NC	1
340		min	0	12	0	3	0	5	4.337e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	5.892e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	4.337e-5	10	NC	1	NC	1
343	M1	max	.008	3	.025	3	.007	5	1.17e-2	1	NC	1	NC	1
344		min	-.009	2	-.022	2	-.004	1	-1.591e-2	3	NC	1	NC	1
345		max	.008	3	.015	3	.01	5	5.601e-3	2	NC	4	NC	1
346		min	-.009	2	-.013	2	-.007	1	-7.871e-3	3	4749.362	3	NC	1
347		max	.008	3	.005	3	.014	5	4.539e-4	5	NC	4	NC	2
348		min	-.009	2	-.004	2	-.009	1	-4.501e-4	1	2460.601	3	7603.186	5
349		max	.008	3	.004	2	.017	5	4.625e-4	5	NC	4	NC	2
350		min	-.009	2	-.003	3	-.01	1	-3.868e-4	1	1753.935	3	4792.892	5
351		max	.008	3	.01	2	.021	5	4.71e-4	5	NC	4	NC	2
352		min	-.009	2	-.009	3	-.011	1	-3.236e-4	1	1391.782	2	3425.886	5
353		max	.008	3	.016	2	.025	5	4.796e-4	5	NC	4	NC	2
354		min	-.009	2	-.015	3	-.01	1	-2.603e-4	1	1182.263	2	2629.482	5
355		max	.008	3	.02	2	.029	5	4.882e-4	5	NC	5	NC	2
356		min	-.009	2	-.019	3	-.009	1	-1.971e-4	1	1052.966	2	2114.658	5
357		max	.008	3	.024	2	.034	5	4.967e-4	5	NC	5	NC	1
358		min	-.009	2	-.021	3	-.007	1	-1.338e-4	1	971.774	2	1758.38	5
359		max	.008	3	.026	2	.039	5	5.053e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.005	1	-7.061e-5	1	923.516	2	1498.477	4
361		max	.008	3	.027	2	.043	5	5.15e-4	4	NC	5	NC	1
362		min	-.009	2	-.023	3	-.003	1	-7.363e-6	1	900.943	2	1285.337	4
363		max	.008	3	.026	2	.048	4	5.381e-4	4	NC	5	NC	1
364		min	-.009	2	-.023	3	0	1	1.287e-5	10	901.423	2	1124.661	4
365		max	.008	3	.025	2	.053	4	5.612e-4	4	NC	5	NC	1
366		min	-.009	2	-.021	3	0	10	1.752e-5	10	925.963	2	1000.821	4
367		max	.008	3	.021	2	.058	4	5.844e-4	4	NC	4	NC	2
368		min	-.009	2	-.018	3	0	10	2.216e-5	10	979.772	2	903.774	4
369		max	.008	3	.017	2	.063	4	6.075e-4	4	NC	4	NC	2
370		min	-.009	2	-.014	3	0	10	2.681e-5	10	1074.902	2	826.822	4
371		max	.008	3	.011	2	.068	4	6.306e-4	4	NC	4	NC	2
372		min	-.009	2	-.009	3	0	10	3.146e-5	10	1237.852	2	765.375	4
373		max	.008	3	.003	2	.072	4	9.129e-4	4	NC	4	NC	2
374		min	-.009	2	-.003	3	0	10	3.487e-5	10	1533.514	2	716.212	4
375		max	.008	3	.004	3	.076	4	7.365e-3	4	NC	4	NC	2
376		min	-.009	2	-.006	2	0	10	-6.274e-5	1	2170.125	2	677.077	4
377		max	.008	3	.012	3	.079	4	8.127e-3	2	NC	4	NC	1
378		min	-.009	2	-.017	2	0	10	-3.839e-3	3	4204.277	2	646.252	4
379		max	.008	3	.02	3	.082	4	1.64e-2	2	NC	1	NC	1
380		min	-.009	2	-.029	2	-.002	1	-7.794e-3	3	NC	1	623.271	4
381	M5	max	.026	3	.08	3	.007	5	1.098e-5	4	NC	1	NC	1
382		min	-.03	2	-.073	2	-.004	1	4.357e-8	2	NC	1	NC	1
383		max	.026	3	.047	3	.01	5	2.279e-4	5	NC	4	NC	1
384		min	-.03	2	-.042	2	-.004	1	-6.786e-5	1	1466.127	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.026	3	.017	3	.014	5	4.412e-4	5	NC	5	NC	1
386		min	-.03	2	-.013	2	-.004	1	-1.345e-4	1	758.622	2	NC	1
387	4	max	.026	3	.012	2	.017	5	4.602e-4	5	NC	5	NC	1
388		min	-.03	2	-.009	3	-.004	1	-1.284e-4	1	529.802	2	NC	1
389	5	max	.026	3	.034	2	.022	5	4.793e-4	5	NC	5	NC	1
390		min	-.03	2	-.03	3	-.004	1	-1.223e-4	1	418.994	2	NC	1
391	6	max	.026	3	.053	2	.026	5	4.983e-4	5	NC	5	NC	1
392		min	-.03	2	-.047	3	-.004	1	-1.161e-4	1	355.635	2	NC	1
393	7	max	.026	3	.068	2	.031	5	5.173e-4	5	NC	5	NC	1
394		min	-.03	2	-.06	3	-.003	1	-1.1e-4	1	316.508	2	NC	1
395	8	max	.026	3	.079	2	.036	5	5.363e-4	5	NC	5	NC	1
396		min	-.03	2	-.069	3	-.003	1	-1.039e-4	1	291.908	2	NC	1
397	9	max	.026	3	.086	2	.041	5	5.553e-4	5	NC	15	NC	1
398		min	-.03	2	-.074	3	-.003	1	-9.776e-5	1	277.247	2	NC	1
399	10	max	.026	3	.089	2	.046	5	5.743e-4	5	NC	15	NC	1
400		min	-.03	2	-.075	3	-.003	1	-9.163e-5	1	270.329	2	NC	1
401	11	max	.026	3	.088	2	.051	5	5.934e-4	5	NC	15	NC	1
402		min	-.03	2	-.073	3	-.003	1	-8.55e-5	1	270.354	2	NC	1
403	12	max	.026	3	.082	2	.055	5	6.124e-4	5	NC	5	NC	1
404		min	-.03	2	-.067	3	-.003	1	-7.937e-5	1	277.615	2	NC	1
405	13	max	.026	3	.072	2	.06	5	6.314e-4	5	NC	5	NC	1
406		min	-.03	2	-.057	3	-.002	1	-7.324e-5	1	293.673	2	NC	1
407	14	max	.025	3	.056	2	.065	4	6.504e-4	5	NC	5	NC	1
408		min	-.03	2	-.044	3	-.002	1	-6.711e-5	1	322.148	2	NC	1
409	15	max	.025	3	.036	2	.069	4	6.694e-4	5	NC	5	NC	1
410		min	-.03	2	-.028	3	-.002	1	-6.098e-5	1	371.007	2	NC	1
411	16	max	.025	3	.01	2	.073	4	9.451e-4	5	NC	5	NC	1
412		min	-.03	2	-.008	3	-.002	1	-5.902e-5	1	459.812	2	NC	1
413	17	max	.025	3	.014	3	.076	4	7.355e-3	4	NC	5	NC	1
414		min	-.03	2	-.021	2	-.002	1	-1.565e-4	1	651.721	2	NC	1
415	18	max	.025	3	.039	3	.079	4	3.773e-3	4	NC	4	NC	1
416		min	-.03	2	-.057	2	-.002	1	-7.988e-5	1	1263.565	2	NC	1
417	19	max	.025	3	.065	3	.082	4	3.218e-6	5	NC	1	NC	1
418		min	-.03	2	-.096	2	-.002	1	-5.249e-7	3	NC	1	NC	1
419	M9	1	max	.008	3	.024	.006	5	1.592e-2	3	NC	1	NC	1
420		min	-.009	2	-.022	2	-.004	1	-1.17e-2	1	NC	1	NC	1
421	2	max	.008	3	.014	3	.006	5	7.861e-3	3	NC	4	NC	1
422		min	-.009	2	-.013	2	0	1	-5.713e-3	1	4750.429	3	NC	1
423	3	max	.008	3	.005	3	.006	4	1.666e-4	1	NC	4	NC	1
424		min	-.009	2	-.004	2	0	3	-4.875e-5	3	2461.162	3	NC	1
425	4	max	.008	3	.004	2	.007	4	1.147e-4	1	NC	4	NC	1
426		min	-.009	2	-.003	3	-.001	3	-5.414e-5	3	1754.312	3	NC	1
427	5	max	.008	3	.01	2	.009	4	6.285e-5	1	NC	4	NC	2
428		min	-.009	2	-.01	3	-.002	3	-5.954e-5	3	1392.653	2	8637.291	14
429	6	max	.008	3	.016	2	.012	4	2.152e-5	2	NC	4	NC	1
430		min	-.009	2	-.015	3	-.003	3	-6.494e-5	3	1182.995	2	6600.517	4
431	7	max	.008	3	.02	2	.015	4	2.037e-5	5	NC	4	NC	1
432		min	-.009	2	-.019	3	-.003	3	-7.033e-5	3	1053.608	2	4525.451	4
433	8	max	.008	3	.024	2	.019	4	3.383e-5	5	NC	5	NC	1
434		min	-.009	2	-.022	3	-.004	3	-9.283e-5	1	972.357	2	3315.458	4
435	9	max	.008	3	.026	2	.023	5	4.729e-5	5	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	3	-1.447e-4	1	924.06	2	2548.561	4
437	10	max	.008	3	.027	2	.028	5	6.075e-5	5	NC	5	NC	1
438		min	-.009	2	-.024	3	-.004	3	-1.966e-4	1	901.463	2	2031.644	4
439	11	max	.008	3	.026	2	.034	5	7.42e-5	5	NC	5	NC	1
440		min	-.009	2	-.023	3	-.005	1	-2.485e-4	1	901.933	2	1666.406	4
441	12	max	.008	3	.025	2	.04	5	8.766e-5	5	NC	5	NC	2





RISA-3D Version 13.0.0      \...\...\PVMMini 60 Cell 1V 30° 100mph 30psf 5.75ft 7-05Page 40



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.126	3	.043	1	6.629e-3	2	NC	5	NC	3
500		min	-.082	4	-.25	2	-.002	10	-4.581e-3	3	622.158	2	2914.179	1
501	4	max	.002	1	.158	3	.065	1	7.761e-3	2	NC	5	NC	3
502		min	-.082	4	-.315	2	0	10	-5.333e-3	3	481.82	2	1997.055	1
503	5	max	.002	1	.17	3	.074	1	8.892e-3	2	NC	5	NC	10
504		min	-.082	4	-.337	2	-.001	10	-6.084e-3	3	447.336	2	1755.809	1
505	6	max	.002	1	.163	3	.068	1	1.002e-2	2	NC	5	NC	5
506		min	-.082	4	-.317	2	-.004	10	-6.836e-3	3	478.005	2	1894.319	1
507	7	max	.002	1	.14	3	.048	1	1.115e-2	2	NC	5	NC	2
508		min	-.082	4	-.264	2	-.008	10	-7.587e-3	3	586.714	2	2600.776	1
509	8	max	.002	1	.108	3	.028	3	1.229e-2	2	NC	5	NC	2
510		min	-.082	4	-.192	2	-.012	10	-8.339e-3	3	842.943	2	5605.274	1
511	9	max	.002	1	.079	3	.027	3	1.342e-2	2	NC	4	NC	1
512		min	-.082	4	-.126	2	-.024	2	-9.09e-3	3	1416.476	2	7293.782	3
513	10	max	.002	1	.065	3	.025	3	1.455e-2	2	NC	4	NC	4
514		min	-.082	4	-.096	2	-.03	2	-9.841e-3	3	2057.252	2	6427.842	2
515	11	max	.002	1	.079	3	.024	3	1.342e-2	2	NC	4	NC	1
516		min	-.082	4	-.126	2	-.024	2	-9.089e-3	3	1416.476	2	8564.522	3
517	12	max	.002	1	.108	3	.024	3	1.229e-2	2	NC	5	NC	2
518		min	-.082	4	-.192	2	-.012	10	-8.336e-3	3	842.943	2	5553.801	1
519	13	max	.002	1	.14	3	.049	1	1.116e-2	2	NC	5	NC	2
520		min	-.082	4	-.264	2	-.008	10	-7.584e-3	3	586.714	2	2595.765	1
521	14	max	.002	1	.163	3	.068	1	1.002e-2	2	NC	5	NC	3
522		min	-.082	4	-.317	2	-.004	10	-6.831e-3	3	478.005	2	1896.88	1
523	15	max	.002	1	.17	3	.074	1	8.893e-3	2	NC	5	NC	3
524		min	-.082	4	-.337	2	-.002	10	-6.078e-3	3	447.336	2	1762.773	1
525	16	max	.002	1	.158	3	.064	1	7.762e-3	2	NC	5	NC	3
526		min	-.082	4	-.315	2	-.005	5	-5.326e-3	3	481.82	2	2010.742	1
527	17	max	.002	1	.126	3	.042	1	6.631e-3	2	NC	5	NC	3
528		min	-.082	4	-.25	2	-.007	5	-4.573e-3	3	622.158	2	2946.179	1
529	18	max	.002	1	.078	3	.016	1	5.5e-3	2	NC	5	NC	2
530		min	-.082	4	-.15	2	-.006	5	-3.82e-3	3	1135.41	2	6847.479	1
531	19	max	.002	1	.02	3	.008	3	4.369e-3	2	NC	1	NC	1
532		min	-.082	4	-.029	2	-.009	2	-3.068e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.862e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.425e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.008	4	8.72e-4	3	NC	1	NC	1
536		min	0	5	-.006	1	0	3	-6.639e-4	5	NC	1	NC	1
537	3	max	0	3	0	5	.016	4	1.358e-3	3	NC	5	NC	1
538		min	-.001	5	-.013	1	-.004	3	-1.054e-3	2	6313.574	1	4826.05	4
539	4	max	0	3	0	5	.026	4	1.844e-3	3	NC	5	NC	9
540		min	-.002	5	-.018	1	-.007	3	-1.55e-3	2	4331.481	1	3079.093	4
541	5	max	0	3	.001	5	.035	4	2.329e-3	3	NC	5	NC	9
542		min	-.003	5	-.024	1	-.012	3	-2.046e-3	2	3379.898	1	2291.85	4
543	6	max	0	3	.001	5	.042	4	2.815e-3	3	NC	5	9079.056	9
544		min	-.003	5	-.028	1	-.018	3	-2.542e-3	2	2844.542	1	1876.486	4
545	7	max	0	3	.002	5	.048	4	3.301e-3	3	NC	5	7188.196	9
546		min	-.004	5	-.032	1	-.023	3	-3.038e-3	2	2522.595	1	1645.941	4
547	8	max	0	3	.002	5	.052	4	3.787e-3	3	NC	5	5985.232	9
548		min	-.005	5	-.034	1	-.029	3	-3.535e-3	2	2329.379	1	1525.999	4
549	9	max	0	3	.002	5	.053	4	4.273e-3	3	NC	5	5192.095	9
550		min	-.005	5	-.036	1	-.033	3	-4.031e-3	2	2225.379	1	1485.386	4
551	10	max	0	3	.003	5	.052	4	4.759e-3	3	NC	5	4667.255	9
552		min	-.006	5	-.037	1	-.037	3	-4.527e-3	2	2192.478	1	1496.22	3
553	11	max	0	3	.003	5	.049	4	5.244e-3	3	NC	5	4336.014	9
554		min	-.007	5	-.036	1	-.04	3	-5.023e-3	2	2225.379	1	1383.74	3
555	12	max	0	3	.003	5	.043	4	5.73e-3	3	NC	5	4161.844	9





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

---

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

---

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.