

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	25° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	140 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 30.77 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.938 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

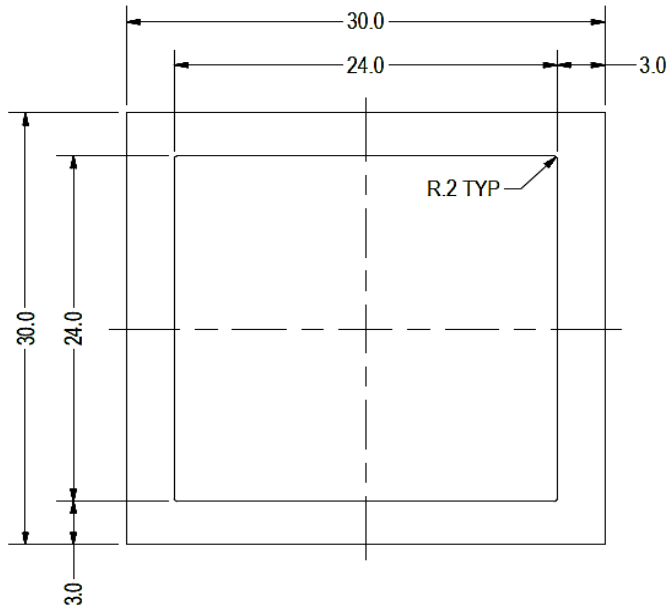
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.458 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

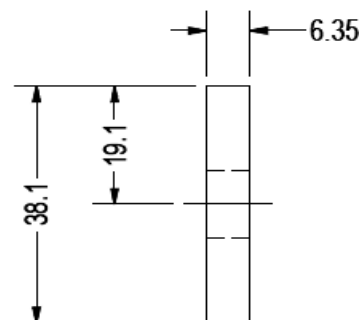
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.762 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	13%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.077 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 39 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

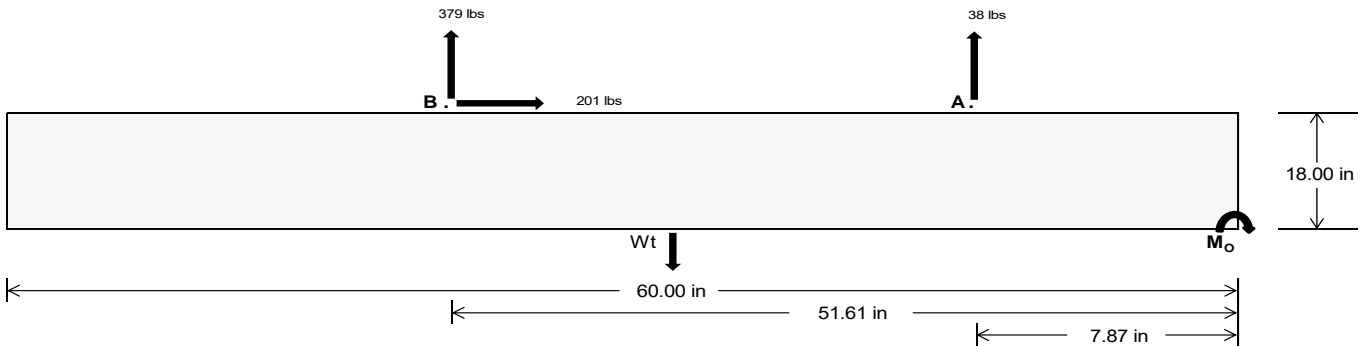
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	171.21	1645.31	k
Compressive Load =	1218.77	1132.45	k
Lateral Load =	1.70	869.34	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23461.4$ in-lbs
Resisting Force Required = 782.05 lbs
S.F. = 1.67
Weight Required = 1303.41 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 200.53 lbs
Friction = 0.4
Weight Required = 501.34 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 200.53 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width	21 in	22 in	23 in	24 in
	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	418 lbs	418 lbs	418 lbs	418 lbs	426 lbs	426 lbs	426 lbs	426 lbs	598 lbs	598 lbs	598 lbs	598 lbs	-76 lbs	-76 lbs	-76 lbs	-76 lbs
F_B	295 lbs	295 lbs	295 lbs	295 lbs	463 lbs	463 lbs	463 lbs	463 lbs	544 lbs	544 lbs	544 lbs	544 lbs	-758 lbs	-758 lbs	-758 lbs	-758 lbs
F_V	38 lbs	38 lbs	38 lbs	38 lbs	359 lbs	359 lbs	359 lbs	359 lbs	294 lbs	294 lbs	294 lbs	294 lbs	-401 lbs	-401 lbs	-401 lbs	-401 lbs
P_{total}	2616 lbs	2707 lbs	2797 lbs	2888 lbs	2793 lbs	2883 lbs	2974 lbs	3064 lbs	3045 lbs	3136 lbs	3226 lbs	3317 lbs	308 lbs	362 lbs	417 lbs	471 lbs
M	295 lbs-ft	295 lbs-ft	295 lbs-ft	295 lbs-ft	489 lbs-ft	489 lbs-ft	489 lbs-ft	489 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft
e	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.18 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.18 ft	0.18 ft	0.17 ft	2.02 ft	1.72 ft	1.49 ft	1.32 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	258.6 psf	256.7 psf	255.0 psf	253.4 psf	252.1 psf	250.5 psf	249.1 psf	247.7 psf	270.4 psf	268.0 psf	265.8 psf	263.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	339.4 psf	333.8 psf	328.8 psf	324.1 psf	386.2 psf	378.5 psf	371.5 psf	365.1 psf	425.5 psf	416.1 psf	407.4 psf	399.5 psf	244.7 psf	168.4 psf	144.0 psf	133.2 psf

Maximum Bearing Pressure = 426 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

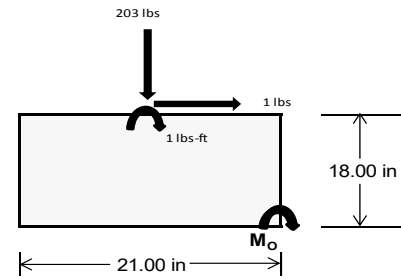
Overturning Check

$M_o = 174.9 \text{ ft-lbs}$
 Resisting Force Required = 199.90 lbs
 S.F. = 1.67
 Weight Required = 333.17 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	59 lbs	149 lbs	56 lbs	203 lbs	586 lbs	199 lbs	17 lbs	43 lbs	16 lbs
F_v	0 lbs	0 lbs	0 lbs	1 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2415 lbs	2505 lbs	2412 lbs	2445 lbs	2829 lbs	2442 lbs	706 lbs	732 lbs	705 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	275.9 sqft	286.2 sqft	275.6 sqft	278.6 sqft	323.0 sqft	278.9 sqft	80.7 sqft	83.7 sqft	80.6 sqft
f_{max}	276.1 psf	286.3 psf	275.7 psf	280.4 psf	323.6 psf	279.3 psf	80.7 psf	83.7 psf	80.6 psf



Maximum Bearing Pressure = 324 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

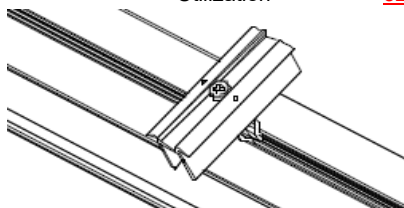
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

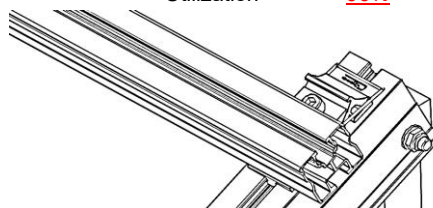
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.635 k
Allowable Uplift =	1.214 k
Utilization =	<u>52%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.074 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

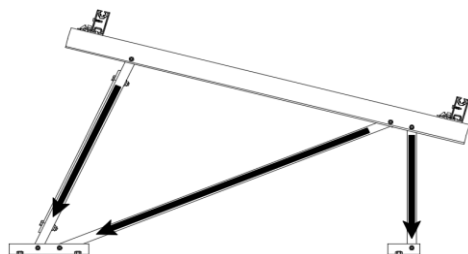
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.938 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

Diagonal Strut

Maximum Axial Load =	0.458 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.116 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.077 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ =	$0.020h_{sx}$
	0.617 in
Max Drift, Δ_{MAX} =	0.01 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$148.425$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$154.13$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.251 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.24 \\ &22.039 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.6 \text{ ksi} \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.24 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.6 \text{ ksi} \end{aligned}$$

3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.5514 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.7972 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 11.5927 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 11.59 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.82 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	177.406	2	267.153	2	0	15	0	9	0	1	0	1
2		min	-216.676	3	-391.953	3	-.139	3	0	3	0	1	0	1
3	N7	max	0	15	333.194	1	-.025	15	0	15	0	1	0	1
4		min	-.126	2	-30.46	3	-.635	1	-.001	1	0	1	0	1
5	N15	max	0	15	937.519	1	.262	1	0	1	0	1	0	1
6		min	-1.304	2	-131.701	3	-.522	3	0	3	0	1	0	1
7	N16	max	611.498	2	871.117	2	0	2	0	1	0	1	0	1
8		min	-668.723	3	-1265.62	3	-63.654	3	0	3	0	1	0	1
9	N23	max	0	15	333.175	1	1.233	1	.002	1	0	1	0	1
10		min	-.126	2	-29.97	3	.036	10	0	10	0	1	0	1
11	N24	max	177.407	2	269.826	2	64.168	3	0	1	0	1	0	1
12		min	-216.996	3	-390.589	3	.001	10	0	3	0	1	0	1
13	Totals:	max	964.754	2	2862.472	1	0	2						
14		min	-1102.577	3	-2240.293	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	238.761	1	.644	4	.223	1	0	15	0	15	0	1
2			min	-353.712	3	.152	15	-.076	3	0	1	0	1	0	1
3		2	max	238.877	1	.598	4	.223	1	0	15	0	15	0	15
4			min	-353.624	3	.141	15	-.076	3	0	1	0	1	0	4
5		3	max	238.994	1	.553	4	.223	1	0	15	0	9	0	15
6			min	-353.537	3	.13	15	-.076	3	0	1	0	3	0	4
7		4	max	239.11	1	.507	4	.223	1	0	15	0	1	0	15
8			min	-353.45	3	.12	15	-.076	3	0	1	0	3	0	4
9		5	max	239.226	1	.461	4	.223	1	0	15	0	1	0	15
10			min	-353.363	3	.109	15	-.076	3	0	1	0	3	0	4
11		6	max	239.343	1	.416	4	.223	1	0	15	0	1	0	15
12			min	-353.275	3	.098	15	-.076	3	0	1	0	3	0	4
13		7	max	239.459	1	.37	4	.223	1	0	15	0	1	0	15
14			min	-353.188	3	.087	15	-.076	3	0	1	0	3	0	4
15		8	max	239.576	1	.324	4	.223	1	0	15	0	1	0	15
16			min	-353.101	3	.077	15	-.076	3	0	1	0	3	0	4
17		9	max	239.692	1	.279	4	.223	1	0	15	0	1	0	15
18			min	-353.013	3	.066	15	-.076	3	0	1	0	3	0	4
19		10	max	239.808	1	.233	4	.223	1	0	15	0	1	0	15
20			min	-352.926	3	.055	15	-.076	3	0	1	0	3	0	4
21		11	max	239.925	1	.187	4	.223	1	0	15	0	1	0	15
22			min	-352.839	3	.044	15	-.076	3	0	1	0	3	0	4
23		12	max	240.041	1	.142	4	.223	1	0	15	0	1	0	15
24			min	-352.751	3	.034	15	-.076	3	0	1	0	3	0	4
25		13	max	240.158	1	.103	2	.223	1	0	15	0	1	0	15
26			min	-352.664	3	.019	12	-.076	3	0	1	0	3	0	4
27		14	max	240.274	1	.067	2	.223	1	0	15	0	1	0	15
28			min	-352.577	3	-.002	3	-.076	3	0	1	0	3	0	4
29		15	max	240.39	1	.031	2	.223	1	0	15	0	1	0	15
30			min	-352.49	3	-.028	3	-.076	3	0	1	0	3	0	4
31		16	max	240.507	1	-.004	2	.223	1	0	15	0	1	0	15
32			min	-352.402	3	-.055	3	-.076	3	0	1	0	3	0	4
33		17	max	240.623	1	-.02	15	.223	1	0	15	0	1	0	15
34			min	-352.315	3	-.087	4	-.076	3	0	1	0	3	0	4
35		18	max	240.74	1	-.031	15	.223	1	0	15	0	1	0	15
36			min	-352.228	3	-.132	4	-.076	3	0	1	0	3	0	4
37		19	max	240.856	1	-.042	15	.223	1	0	15	0	1	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-352.14	3	-.178	4	-.076	3	0	1	0	3	0	4
39	M3	1	max	127.625	2	1.779	4	-.009	15	0	15	0	1	0	4
40			min	-126.265	3	.418	15	-.232	1	0	1	0	15	0	15
41		2	max	127.556	2	1.601	4	-.009	15	0	15	0	1	0	2
42			min	-126.316	3	.377	15	-.232	1	0	1	0	15	0	12
43		3	max	127.488	2	1.424	4	-.009	15	0	15	0	1	0	2
44			min	-126.368	3	.335	15	-.232	1	0	1	0	15	0	3
45		4	max	127.419	2	1.247	4	-.009	15	0	15	0	1	0	15
46			min	-126.419	3	.293	15	-.232	1	0	1	0	15	0	4
47		5	max	127.351	2	1.07	4	-.009	15	0	15	0	1	0	15
48			min	-126.471	3	.252	15	-.232	1	0	1	0	15	0	4
49		6	max	127.282	2	.893	4	-.009	15	0	15	0	1	0	15
50			min	-126.522	3	.21	15	-.232	1	0	1	0	15	0	4
51		7	max	127.213	2	.715	4	-.009	15	0	15	0	1	0	15
52			min	-126.573	3	.168	15	-.232	1	0	1	0	15	0	4
53		8	max	127.145	2	.538	4	-.009	15	0	15	0	1	0	15
54			min	-126.625	3	.127	15	-.232	1	0	1	0	15	-.001	4
55		9	max	127.076	2	.361	4	-.009	15	0	15	0	1	0	15
56			min	-126.676	3	.085	15	-.232	1	0	1	0	15	-.001	4
57		10	max	127.008	2	.184	4	-.009	15	0	15	0	1	0	15
58			min	-126.728	3	.043	15	-.232	1	0	1	0	15	-.001	4
59		11	max	126.939	2	.027	2	-.009	15	0	15	0	1	0	15
60			min	-126.779	3	-.021	3	-.232	1	0	1	0	15	-.001	4
61		12	max	126.87	2	-.04	15	-.009	15	0	15	0	1	0	15
62			min	-126.831	3	-.171	4	-.232	1	0	1	0	15	-.001	4
63		13	max	126.802	2	-.081	15	-.009	15	0	15	0	1	0	15
64			min	-126.882	3	-.348	4	-.232	1	0	1	0	10	-.001	4
65		14	max	126.733	2	-.123	15	-.009	15	0	15	0	1	0	15
66			min	-126.934	3	-.525	4	-.232	1	0	1	0	10	-.001	4
67		15	max	126.665	2	-.165	15	-.009	15	0	15	0	1	0	15
68			min	-126.985	3	-.702	4	-.232	1	0	1	0	10	0	4
69		16	max	126.596	2	-.206	15	-.009	15	0	15	0	15	0	15
70			min	-127.037	3	-.879	4	-.232	1	0	1	0	1	0	4
71		17	max	126.527	2	-.248	15	-.009	15	0	15	0	15	0	15
72			min	-127.088	3	-1.057	4	-.232	1	0	1	0	1	0	4
73		18	max	126.459	2	-.29	15	-.009	15	0	15	0	15	0	15
74			min	-127.139	3	-1.234	4	-.232	1	0	1	0	1	0	4
75		19	max	126.39	2	-.331	15	-.009	15	0	15	0	15	0	1
76			min	-127.191	3	-1.411	4	-.232	1	0	1	0	1	0	1
77	M4	1	max	332.03	1	0	1	-.025	15	0	1	0	3	0	1
78			min	-31.334	3	0	1	-.672	1	0	1	0	2	0	1
79		2	max	332.094	1	0	1	-.025	15	0	1	0	15	0	1
80			min	-31.285	3	0	1	-.672	1	0	1	0	1	0	1
81		3	max	332.159	1	0	1	-.025	15	0	1	0	15	0	1
82			min	-31.237	3	0	1	-.672	1	0	1	0	1	0	1
83		4	max	332.224	1	0	1	-.025	15	0	1	0	15	0	1
84			min	-31.188	3	0	1	-.672	1	0	1	0	1	0	1
85		5	max	332.288	1	0	1	-.025	15	0	1	0	15	0	1
86			min	-31.14	3	0	1	-.672	1	0	1	0	1	0	1
87		6	max	332.353	1	0	1	-.025	15	0	1	0	15	0	1
88			min	-31.091	3	0	1	-.672	1	0	1	0	1	0	1
89		7	max	332.418	1	0	1	-.025	15	0	1	0	15	0	1
90			min	-31.043	3	0	1	-.672	1	0	1	0	1	0	1
91		8	max	332.482	1	0	1	-.025	15	0	1	0	15	0	1
92			min	-30.994	3	0	1	-.672	1	0	1	0	1	0	1
93		9	max	332.547	1	0	1	-.025	15	0	1	0	15	0	1
94			min	-30.946	3	0	1	-.672	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	332.612	1	0	1	-.025	15	0	1	0	15	0	1
96		min	-30.897	3	0	1	-.672	1	0	1	0	1	0	1
97	11	max	332.677	1	0	1	-.025	15	0	1	0	15	0	1
98		min	-30.849	3	0	1	-.672	1	0	1	0	1	0	1
99	12	max	332.741	1	0	1	-.025	15	0	1	0	15	0	1
100		min	-30.8	3	0	1	-.672	1	0	1	0	1	0	1
101	13	max	332.806	1	0	1	-.025	15	0	1	0	15	0	1
102		min	-30.751	3	0	1	-.672	1	0	1	0	1	0	1
103	14	max	332.871	1	0	1	-.025	15	0	1	0	15	0	1
104		min	-30.703	3	0	1	-.672	1	0	1	0	1	0	1
105	15	max	332.935	1	0	1	-.025	15	0	1	0	15	0	1
106		min	-30.654	3	0	1	-.672	1	0	1	0	1	0	1
107	16	max	333	1	0	1	-.025	15	0	1	0	15	0	1
108		min	-30.606	3	0	1	-.672	1	0	1	0	1	0	1
109	17	max	333.065	1	0	1	-.025	15	0	1	0	15	0	1
110		min	-30.557	3	0	1	-.672	1	0	1	0	1	0	1
111	18	max	333.13	1	0	1	-.025	15	0	1	0	15	0	1
112		min	-30.509	3	0	1	-.672	1	0	1	-.001	1	0	1
113	19	max	333.194	1	0	1	-.025	15	0	1	0	15	0	1
114		min	-30.46	3	0	1	-.672	1	0	1	-.001	1	0	1
115	M6	1	max	759.914	1	.643	.058	1	0	3	0	3	0	1
116		min	-1115.805	3	.151	15	-.225	3	0	2	0	2	0	1
117	2	max	760.03	1	.597	4	.058	1	0	3	0	3	0	15
118		min	-1115.718	3	.141	15	-.225	3	0	2	0	2	0	4
119	3	max	760.146	1	.551	4	.058	1	0	3	0	3	0	15
120		min	-1115.631	3	.13	15	-.225	3	0	2	0	2	0	4
121	4	max	760.263	1	.506	4	.058	1	0	3	0	3	0	15
122		min	-1115.543	3	.119	15	-.225	3	0	2	0	2	0	4
123	5	max	760.379	1	.46	4	.058	1	0	3	0	1	0	15
124		min	-1115.456	3	.108	15	-.225	3	0	2	0	2	0	4
125	6	max	760.496	1	.416	2	.058	1	0	3	0	1	0	15
126		min	-1115.369	3	.098	15	-.225	3	0	2	0	3	0	4
127	7	max	760.612	1	.38	2	.058	1	0	3	0	1	0	15
128		min	-1115.282	3	.087	12	-.225	3	0	2	0	3	0	4
129	8	max	760.728	1	.345	2	.058	1	0	3	0	1	0	15
130		min	-1115.194	3	.069	12	-.225	3	0	2	0	3	0	4
131	9	max	760.845	1	.309	2	.058	1	0	3	0	1	0	15
132		min	-1115.107	3	.051	12	-.225	3	0	2	0	3	0	4
133	10	max	760.961	1	.273	2	.058	1	0	3	0	1	0	15
134		min	-1115.02	3	.034	12	-.225	3	0	2	0	3	0	4
135	11	max	761.078	1	.238	2	.058	1	0	3	0	1	0	15
136		min	-1114.932	3	.014	3	-.225	3	0	2	0	3	0	2
137	12	max	761.194	1	.202	2	.058	1	0	3	0	1	0	15
138		min	-1114.845	3	-.013	3	-.225	3	0	2	0	3	0	2
139	13	max	761.31	1	.167	2	.058	1	0	3	0	1	0	12
140		min	-1114.758	3	-.039	3	-.225	3	0	2	0	3	0	2
141	14	max	761.427	1	.131	2	.058	1	0	3	0	1	0	12
142		min	-1114.67	3	-.066	3	-.225	3	0	2	0	3	0	2
143	15	max	761.543	1	.095	2	.058	1	0	3	0	1	0	12
144		min	-1114.583	3	-.093	3	-.225	3	0	2	0	3	0	2
145	16	max	761.66	1	.06	2	.058	1	0	3	0	1	0	12
146		min	-1114.496	3	-.119	3	-.225	3	0	2	0	3	0	2
147	17	max	761.776	1	.024	2	.058	1	0	3	0	1	0	12
148		min	-1114.409	3	-.146	3	-.225	3	0	2	0	3	0	2
149	18	max	761.893	1	-.011	2	.058	1	0	3	0	1	0	12
150		min	-1114.321	3	-.173	3	-.225	3	0	2	0	3	0	2
151	19	max	762.009	1	-.042	15	.058	1	0	3	0	1	0	12



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1114.234	3	-.199	3	-.225	3	0	2	0	3	0	2
153	M7	1	max	457.514	2	1.78	4	.016	3	0	1	0	1	2
154		min	-368.333	3	.419	15	-.008	1	0	3	0	3	0	12
155		2	max	457.445	2	1.603	4	.016	3	0	1	0	1	2
156		min	-368.384	3	.377	15	-.008	1	0	3	0	3	0	3
157		3	max	457.377	2	1.425	4	.016	3	0	1	0	1	2
158		min	-368.436	3	.335	15	-.008	1	0	3	0	3	0	3
159		4	max	457.308	2	1.248	4	.016	3	0	1	0	1	2
160		min	-368.487	3	.294	15	-.008	1	0	3	0	3	0	3
161		5	max	457.24	2	1.071	4	.016	3	0	1	0	1	15
162		min	-368.539	3	.252	15	-.008	1	0	3	0	3	0	3
163		6	max	457.171	2	.894	4	.016	3	0	1	0	1	15
164		min	-368.59	3	.21	15	-.008	1	0	3	0	3	0	4
165		7	max	457.102	2	.717	4	.016	3	0	1	0	1	15
166		min	-368.641	3	.169	15	-.008	1	0	3	0	3	0	4
167		8	max	457.034	2	.539	4	.016	3	0	1	0	1	15
168		min	-368.693	3	.127	15	-.008	1	0	3	0	3	-.001	4
169		9	max	456.965	2	.362	4	.016	3	0	1	0	1	15
170		min	-368.744	3	.085	15	-.008	1	0	3	0	3	-.001	4
171		10	max	456.897	2	.216	2	.016	3	0	1	0	1	15
172		min	-368.796	3	.025	12	-.008	1	0	3	0	3	-.001	4
173		11	max	456.828	2	.078	2	.016	3	0	1	0	1	15
174		min	-368.847	3	-.074	3	-.008	1	0	3	0	3	-.001	4
175		12	max	456.759	2	-.04	15	.016	3	0	1	0	1	15
176		min	-368.899	3	-.177	3	-.008	1	0	3	0	3	-.001	4
177		13	max	456.691	2	-.081	15	.016	3	0	1	0	1	15
178		min	-368.95	3	-.347	4	-.008	1	0	3	0	3	-.001	4
179		14	max	456.622	2	-.123	15	.016	3	0	1	0	1	15
180		min	-369.002	3	-.524	4	-.008	1	0	3	0	3	-.001	4
181		15	max	456.554	2	-.165	15	.016	3	0	1	0	1	15
182		min	-369.053	3	-.701	4	-.008	1	0	3	0	3	0	4
183		16	max	456.485	2	-.206	15	.016	3	0	1	0	1	15
184		min	-369.105	3	-.878	4	-.008	1	0	3	0	3	0	4
185		17	max	456.416	2	-.248	15	.016	3	0	1	0	1	15
186		min	-369.156	3	-1.055	4	-.008	1	0	3	0	3	0	4
187		18	max	456.348	2	-.289	15	.016	3	0	1	0	1	15
188		min	-369.207	3	-1.233	4	-.008	1	0	3	0	3	0	4
189		19	max	456.279	2	-.331	15	.016	3	0	1	0	1	1
190		min	-369.259	3	-1.41	4	-.008	1	0	3	0	3	0	1
191	M8	1	max	936.354	1	0	1	.309	1	0	1	0	2	1
192		min	-132.574	3	0	1	-.511	3	0	1	0	1	0	1
193		2	max	936.419	1	0	1	.309	1	0	1	0	1	1
194		min	-132.526	3	0	1	-.511	3	0	1	0	3	0	1
195		3	max	936.484	1	0	1	.309	1	0	1	0	1	1
196		min	-132.477	3	0	1	-.511	3	0	1	0	3	0	1
197		4	max	936.548	1	0	1	.309	1	0	1	0	1	1
198		min	-132.429	3	0	1	-.511	3	0	1	0	3	0	1
199		5	max	936.613	1	0	1	.309	1	0	1	0	1	1
200		min	-132.38	3	0	1	-.511	3	0	1	0	3	0	1
201		6	max	936.678	1	0	1	.309	1	0	1	0	1	1
202		min	-132.332	3	0	1	-.511	3	0	1	0	3	0	1
203		7	max	936.742	1	0	1	.309	1	0	1	0	1	1
204		min	-132.283	3	0	1	-.511	3	0	1	0	3	0	1
205		8	max	936.807	1	0	1	.309	1	0	1	0	1	1
206		min	-132.235	3	0	1	-.511	3	0	1	0	3	0	1
207		9	max	936.872	1	0	1	.309	1	0	1	0	1	1
208		min	-132.186	3	0	1	-.511	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	936.937	1	0	1	.309	1	0	1	0	1	0	1
210			min	-132.137	3	0	1	-.511	3	0	1	0	3	0	1
211		11	max	937.001	1	0	1	.309	1	0	1	0	1	0	1
212			min	-132.089	3	0	1	-.511	3	0	1	0	3	0	1
213		12	max	937.066	1	0	1	.309	1	0	1	0	1	0	1
214			min	-132.04	3	0	1	-.511	3	0	1	0	3	0	1
215		13	max	937.131	1	0	1	.309	1	0	1	0	1	0	1
216			min	-131.992	3	0	1	-.511	3	0	1	0	3	0	1
217		14	max	937.195	1	0	1	.309	1	0	1	0	1	0	1
218			min	-131.943	3	0	1	-.511	3	0	1	0	3	0	1
219		15	max	937.26	1	0	1	.309	1	0	1	0	1	0	1
220			min	-131.895	3	0	1	-.511	3	0	1	0	3	0	1
221		16	max	937.325	1	0	1	.309	1	0	1	0	1	0	1
222			min	-131.846	3	0	1	-.511	3	0	1	0	3	0	1
223		17	max	937.389	1	0	1	.309	1	0	1	0	1	0	1
224			min	-131.798	3	0	1	-.511	3	0	1	0	3	0	1
225		18	max	937.454	1	0	1	.309	1	0	1	0	1	0	1
226			min	-131.749	3	0	1	-.511	3	0	1	0	3	0	1
227		19	max	937.519	1	0	1	.309	1	0	1	0	1	0	1
228			min	-131.701	3	0	1	-.511	3	0	1	0	3	0	1
229	M10	1	max	240.753	1	.644	4	-.003	15	0	1	0	1	0	1
230			min	-312.19	3	.152	15	-.113	1	0	3	0	3	0	1
231		2	max	240.869	1	.598	4	-.003	15	0	1	0	1	0	15
232			min	-312.103	3	.141	15	-.113	1	0	3	0	3	0	4
233		3	max	240.986	1	.552	4	-.003	15	0	1	0	1	0	15
234			min	-312.015	3	.13	15	-.113	1	0	3	0	3	0	4
235		4	max	241.102	1	.507	4	-.003	15	0	1	0	1	0	15
236			min	-311.928	3	.119	15	-.113	1	0	3	0	3	0	4
237		5	max	241.219	1	.461	4	-.003	15	0	1	0	1	0	15
238			min	-311.841	3	.109	15	-.113	1	0	3	0	3	0	4
239		6	max	241.335	1	.415	4	-.003	15	0	1	0	1	0	15
240			min	-311.753	3	.098	15	-.113	1	0	3	0	3	0	4
241		7	max	241.451	1	.37	4	-.003	15	0	1	0	1	0	15
242			min	-311.666	3	.087	15	-.113	1	0	3	0	3	0	4
243		8	max	241.568	1	.324	4	-.003	15	0	1	0	9	0	15
244			min	-311.579	3	.077	15	-.113	1	0	3	0	3	0	4
245		9	max	241.684	1	.278	4	-.003	15	0	1	0	15	0	15
246			min	-311.491	3	.066	15	-.113	1	0	3	0	3	0	4
247		10	max	241.801	1	.233	4	-.003	15	0	1	0	15	0	15
248			min	-311.404	3	.055	15	-.113	1	0	3	0	3	0	4
249		11	max	241.917	1	.187	4	-.003	15	0	1	0	15	0	15
250			min	-311.317	3	.044	15	-.113	1	0	3	0	3	0	4
251		12	max	242.033	1	.141	4	-.003	15	0	1	0	15	0	15
252			min	-311.23	3	.034	15	-.113	1	0	3	0	3	0	4
253		13	max	242.15	1	.103	2	-.003	15	0	1	0	15	0	15
254			min	-311.142	3	.023	15	-.113	1	0	3	0	3	0	4
255		14	max	242.266	1	.067	2	-.003	15	0	1	0	15	0	15
256			min	-311.055	3	.012	15	-.113	1	0	3	0	3	0	4
257		15	max	242.383	1	.031	2	-.003	15	0	1	0	15	0	15
258			min	-310.968	3	-.004	9	-.113	1	0	3	0	3	0	4
259		16	max	242.499	1	-.004	2	-.003	15	0	1	0	15	0	15
260			min	-310.88	3	-.041	4	-.113	1	0	3	0	3	0	4
261		17	max	242.615	1	-.02	15	-.003	15	0	1	0	15	0	15
262			min	-310.793	3	-.087	4	-.113	1	0	3	0	3	0	4
263		18	max	242.732	1	-.031	15	-.003	15	0	1	0	15	0	15
264			min	-310.706	3	-.132	4	-.113	1	0	3	0	3	0	4
265		19	max	242.848	1	-.042	15	-.003	15	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-310.618	3	-.178	4	-.113	1	0	3	0	1	0	4
267		1	max	127.162	2	1.779	4	.253	1	0	3	0	3	0	4
268			min	-126.926	3	.418	15	-.031	3	0	10	0	1	0	15
269		2	max	127.093	2	1.601	4	.253	1	0	3	0	3	0	2
270			min	-126.977	3	.377	15	-.031	3	0	10	0	1	0	12
271		3	max	127.025	2	1.424	4	.253	1	0	3	0	3	0	2
272			min	-127.029	3	.335	15	-.031	3	0	10	0	1	0	3
273		4	max	126.956	2	1.247	4	.253	1	0	3	0	3	0	15
274			min	-127.08	3	.293	15	-.031	3	0	10	0	1	0	3
275		5	max	126.888	2	1.07	4	.253	1	0	3	0	3	0	15
276			min	-127.132	3	.252	15	-.031	3	0	10	0	1	0	4
277		6	max	126.819	2	.893	4	.253	1	0	3	0	3	0	15
278			min	-127.183	3	.21	15	-.031	3	0	10	0	1	0	4
279		7	max	126.75	2	.715	4	.253	1	0	3	0	3	0	15
280			min	-127.235	3	.168	15	-.031	3	0	10	0	1	0	4
281		8	max	126.682	2	.538	4	.253	1	0	3	0	3	0	15
282			min	-127.286	3	.127	15	-.031	3	0	10	0	1	-.001	4
283	9	max	126.613	2	.361	4	.253	1	0	3	0	3	0	15	
284		min	-127.338	3	.085	15	-.031	3	0	10	0	1	-.001	4	
285	10	max	126.545	2	.184	4	.253	1	0	3	0	3	0	15	
286		min	-127.389	3	.043	15	-.031	3	0	10	0	1	-.001	4	
287	11	max	126.476	2	.027	2	.253	1	0	3	0	3	0	15	
288		min	-127.441	3	-.035	3	-.031	3	0	10	0	1	-.001	4	
289	12	max	126.407	2	-.04	15	.253	1	0	3	0	3	0	15	
290		min	-127.492	3	-.171	4	-.031	3	0	10	0	1	-.001	4	
291	13	max	126.339	2	-.081	15	.253	1	0	3	0	3	0	15	
292		min	-127.543	3	-.348	4	-.031	3	0	10	0	1	-.001	4	
293	14	max	126.27	2	-.123	15	.253	1	0	3	0	3	0	15	
294		min	-127.595	3	-.525	4	-.031	3	0	10	0	1	-.001	4	
295	15	max	126.202	2	-.165	15	.253	1	0	3	0	3	0	15	
296		min	-127.646	3	-.702	4	-.031	3	0	10	0	2	0	4	
297	16	max	126.133	2	-.206	15	.253	1	0	3	0	3	0	15	
298		min	-127.698	3	-.879	4	-.031	3	0	10	0	10	0	4	
299	17	max	126.064	2	-.248	15	.253	1	0	3	0	3	0	15	
300		min	-127.749	3	-1.057	4	-.031	3	0	10	0	10	0	4	
301	18	max	125.996	2	-.29	15	.253	1	0	3	0	3	0	15	
302		min	-127.801	3	-1.234	4	-.031	3	0	10	0	10	0	4	
303	19	max	125.927	2	-.331	15	.253	1	0	3	0	3	0	1	
304		min	-127.852	3	-1.411	4	-.031	3	0	10	0	10	0	1	
305	M12	1	max	332.01	1	0	1	1.305	1	0	1	0	2	0	1
306		min	-30.844	3	0	1	.037	10	0	1	0	3	0	1	
307	2	max	332.075	1	0	1	1.305	1	0	1	0	1	0	1	
308		min	-30.795	3	0	1	.037	10	0	1	0	15	0	1	
309	3	max	332.14	1	0	1	1.305	1	0	1	0	1	0	1	
310		min	-30.747	3	0	1	.037	10	0	1	0	15	0	1	
311	4	max	332.205	1	0	1	1.305	1	0	1	0	1	0	1	
312		min	-30.698	3	0	1	.037	10	0	1	0	15	0	1	
313	5	max	332.269	1	0	1	1.305	1	0	1	0	1	0	1	
314		min	-30.65	3	0	1	.037	10	0	1	0	15	0	1	
315	6	max	332.334	1	0	1	1.305	1	0	1	0	1	0	1	
316		min	-30.601	3	0	1	.037	10	0	1	0	15	0	1	
317	7	max	332.399	1	0	1	1.305	1	0	1	0	1	0	1	
318		min	-30.553	3	0	1	.037	10	0	1	0	15	0	1	
319	8	max	332.463	1	0	1	1.305	1	0	1	0	1	0	1	
320		min	-30.504	3	0	1	.037	10	0	1	0	15	0	1	
321	9	max	332.528	1	0	1	1.305	1	0	1	0	1	0	1	
322		min	-30.456	3	0	1	.037	10	0	1	0	10	0	1	







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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	58.52	3	3.398	9	26.361	1	0	1	.009	3	.13	2
438			min	-6.026	10	-20.521	2	-1.933	3	0	15	0	1	-.116	3
439		11	max	58.609	3	3.187	9	26.361	1	0	1	.008	3	.135	2
440			min	-5.927	10	-20.774	2	-1.933	3	0	15	0	15	-.112	3
441		12	max	58.697	3	2.976	9	26.361	1	0	1	.011	1	.139	2
442			min	-5.829	10	-21.027	2	-1.933	3	0	15	0	15	-.108	3
443		13	max	58.786	3	2.765	9	26.361	1	0	1	.017	1	.144	2
444			min	-5.731	10	-21.28	2	-1.933	3	0	15	0	15	-.104	3
445		14	max	58.874	3	2.554	9	26.361	1	0	1	.023	1	.149	2
446			min	-5.632	10	-21.533	2	-1.933	3	0	15	0	15	-.1	3
447		15	max	58.963	3	2.343	9	26.361	1	0	1	.029	1	.153	2
448			min	-5.534	10	-21.786	2	-1.933	3	0	15	.001	15	-.096	3
449		16	max	84.317	2	76.022	2	26.589	1	0	15	.035	1	.157	2
450			min	-20.018	3	-120.058	3	-1.955	3	0	1	.001	15	-.091	3
451		17	max	84.435	2	75.769	2	26.589	1	0	15	.04	1	.141	2
452			min	-19.929	3	-120.248	3	-1.955	3	0	1	.002	15	-.064	3
453		18	max	-3.231	15	325.915	2	27.896	1	0	2	.046	1	.071	2
454			min	-86.911	1	-148.427	3	-1.542	3	0	3	.002	15	-.032	3
455		19	max	-3.195	15	325.662	2	27.896	1	0	2	.053	1	0	2
456			min	-86.793	1	-148.617	3	-1.542	3	0	3	.002	15	0	3
457	M13	1	max	61.373	3	240.341	1	-3.196	15	0	2	.053	1	0	1
458			min	1.055	15	-333.36	3	-86.803	1	0	3	.002	15	0	3
459		2	max	61.373	3	170.405	1	-2.433	15	0	2	.012	1	.15	3
460			min	1.055	15	-236.094	3	-65.808	1	0	3	0	10	-.108	1
461		3	max	61.373	3	100.469	1	-1.671	15	0	2	.007	3	.249	3
462			min	1.055	15	-138.828	3	-44.813	1	0	3	-.017	1	-.18	1
463		4	max	61.373	3	30.532	1	-.908	15	0	2	.004	3	.297	3
464			min	1.055	15	-41.561	3	-23.817	1	0	3	-.035	1	-.214	1
465		5	max	61.373	3	55.705	3	1.035	10	0	2	.002	3	.293	3
466			min	1.055	15	-39.404	1	-3.407	3	0	3	-.042	1	-.212	1
467		6	max	61.373	3	152.972	3	18.174	1	0	2	0	3	.238	3
468			min	1.055	15	-109.34	1	-2.297	3	0	3	-.038	1	-.173	1
469		7	max	61.373	3	250.238	3	39.169	1	0	2	0	3	.132	3
470			min	1.055	15	-179.277	1	-1.188	3	0	3	-.023	1	-.097	1
471		8	max	61.373	3	347.504	3	60.164	1	0	2	.004	2	.016	1
472			min	1.055	15	-249.213	1	-.078	3	0	3	0	3	-.026	3
473		9	max	61.373	3	444.771	3	81.16	1	0	2	.041	1	.166	1
474			min	1.055	15	-319.149	1	.901	12	0	3	0	3	-.235	3
475		10	max	61.373	3	542.037	3	102.155	1	0	2	.089	1	.353	1
476			min	1.055	15	-389.086	1	1.641	12	0	3	-.007	3	-.496	3
477		11	max	27.081	1	319.149	1	-.467	3	0	3	.04	1	.166	1
478			min	1.037	15	-444.771	3	-80.921	1	0	2	-.008	3	-.235	3
479		12	max	27.081	1	249.213	1	.643	3	0	3	.004	2	.016	1
480			min	1.037	15	-347.504	3	-59.926	1	0	2	-.007	3	-.026	3
481		13	max	27.081	1	179.277	1	1.752	3	0	3	0	15	.132	3
482			min	1.037	15	-250.238	3	-38.93	1	0	2	-.023	1	-.097	1
483		14	max	27.081	1	109.34	1	2.862	3	0	3	-.001	15	.238	3
484			min	1.037	15	-152.972	3	-17.935	1	0	2	-.038	1	-.173	1
485		15	max	27.081	1	39.404	1	3.972	3	0	3	-.002	15	.293	3
486			min	1.037	15	-55.705	3	-1.035	10	0	2	-.042	1	-.212	1
487		16	max	27.081	1	41.561	3	24.056	1	0	3	0	12	.297	3
488			min	1.037	15	-30.532	1	.92	15	0	2	-.035	1	-.214	1
489		17	max	27.081	1	138.828	3	45.051	1	0	3	.002	3	.249	3
490			min	1.037	15	-100.469	1	1.682	15	0	2	-.017	1	-.18	1
491		18	max	27.081	1	236.094	3	66.047	1	0	3	.013	1	.15	3
492			min	1.037	15	-170.405	1	2.445	15	0	2	0	10	-.108	1
493		19	max	27.081	1	333.36	3	87.042	1	0	3	.053	1	0	1



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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	1.037	15	-240.342	1	3.207	15	0	2	.002	15	0	3
495		max	1.544	3	325.781	2	-3.195	15	0	3	.053	1	0	2
496		min	-27.838	1	-148.637	3	-86.799	1	0	2	.002	15	0	3
497		2 max	1.544	3	230.985	2	-2.433	15	0	3	.012	1	.067	3
498		min	-27.838	1	-105.723	3	-65.804	1	0	2	0	10	-.147	2
499		3 max	1.544	3	136.189	2	-1.67	15	0	3	0	12	.112	3
500		min	-27.838	1	-62.809	3	-44.809	1	0	2	-.017	1	-.244	2
501		4 max	1.544	3	41.393	2	-.908	15	0	3	-.001	15	.133	3
502		min	-27.838	1	-19.894	3	-23.813	1	0	2	-.035	1	-.291	2
503		5 max	1.544	3	23.02	3	1.029	10	0	3	-.002	15	.133	3
504		min	-27.838	1	-53.403	2	-2.818	1	0	2	-.042	1	-.287	2
505		6 max	1.544	3	65.934	3	18.178	1	0	3	-.001	15	.109	3
506		min	-27.838	1	-148.199	2	-1.18	3	0	2	-.038	1	-.234	2
507		7 max	1.544	3	108.849	3	39.173	1	0	3	0	15	.063	3
508		min	-27.838	1	-242.994	2	-.07	3	0	2	-.023	1	-.131	2
509		8 max	1.544	3	151.763	3	60.168	1	0	3	.004	2	.022	2
510		min	-27.838	1	-337.79	2	.832	12	0	2	-.005	3	-.006	3
511		9 max	1.544	3	194.677	3	81.164	1	0	3	.041	1	.225	2
512		min	-27.838	1	-432.586	2	1.571	12	0	2	-.004	3	-.097	3
513		10 max	-1.064	15	-9.103	15	102.159	1	0	15	.089	1	.479	2
514		min	-27.838	1	-527.382	2	-4.067	3	0	2	-.003	3	-.211	3
515		11 max	-1.062	15	432.586	2	-2.053	12	0	2	.04	1	.225	2
516		min	-27.758	1	-194.677	3	-80.925	1	0	3	0	3	-.097	3
517		12 max	-1.062	15	337.79	2	-1.314	12	0	2	.004	2	.022	2
518		min	-27.758	1	-151.763	3	-59.93	1	0	3	0	3	-.006	3
519		13 max	-1.062	15	242.994	2	-.574	12	0	2	0	15	.063	3
520		min	-27.758	1	-108.848	3	-38.935	1	0	3	-.023	1	-.131	2
521		14 max	-1.062	15	148.199	2	.372	3	0	2	-.001	12	.109	3
522		min	-27.758	1	-65.934	3	-17.939	1	0	3	-.038	1	-.234	2
523		15 max	-1.062	15	53.403	2	3.056	1	0	2	0	12	.133	3
524		min	-27.758	1	-23.02	3	-1.029	10	0	3	-.042	1	-.287	2
525		16 max	-1.062	15	19.894	3	24.051	1	0	2	0	3	.133	3
526		min	-27.758	1	-41.393	2	.919	15	0	3	-.035	1	-.291	2
527		17 max	-1.062	15	62.809	3	45.047	1	0	2	.001	3	.112	3
528		min	-27.758	1	-136.189	2	1.681	15	0	3	-.017	1	-.244	2
529		18 max	-1.062	15	105.723	3	66.042	1	0	2	.013	1	.067	3
530		min	-27.758	1	-230.985	2	2.444	15	0	3	0	10	-.147	2
531		19 max	-1.062	15	148.637	3	87.038	1	0	2	.053	1	0	2
532		min	-27.758	1	-325.781	2	3.206	15	0	3	.002	15	0	3
533		M15	1 max	0	.978	3	.096	3	0	1	0	1	0	1
534			min	-75.572	3	0	0	1	0	3	0	3	0	1
535		2 max	0	1	.87	3	.096	3	0	1	0	1	0	1
536		min	-75.637	3	0	1	0	1	0	3	0	3	0	3
537		3 max	0	1	.761	3	.096	3	0	1	0	1	0	1
538		min	-75.702	3	0	1	0	1	0	3	0	3	0	3
539		4 max	0	1	.652	3	.096	3	0	1	0	1	0	1
540		min	-75.768	3	0	1	0	1	0	3	0	3	0	3
541		5 max	0	1	.544	3	.096	3	0	1	0	1	0	1
542		min	-75.833	3	0	1	0	1	0	3	0	3	0	3
543		6 max	0	1	.435	3	.096	3	0	1	0	1	0	1
544		min	-75.898	3	0	1	0	1	0	3	0	3	-.001	3
545		7 max	0	1	.326	3	.096	3	0	1	0	3	0	1
546		min	-75.963	3	0	1	0	1	0	3	0	1	-.001	3
547		8 max	0	1	.217	3	.096	3	0	1	0	3	0	1
548		min	-76.028	3	0	1	0	1	0	3	0	1	-.001	3
549		9 max	0	1	.109	3	.096	3	0	1	0	3	0	1
550		min	-76.094	3	0	1	0	1	0	3	0	1	-.001	3



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.096	3	0	1	0	3	0	1
552		min	-76.159	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.096	3	0	1	0	3	0	1
554		min	-76.224	3	-.109	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.096	3	0	1	0	3	0	1
556		min	-76.289	3	-.217	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.096	3	0	1	0	3	0	1
558		min	-76.354	3	-.326	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.096	3	0	1	0	3	0	1
560		min	-76.42	3	-.435	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.096	3	0	1	0	3	0	1
562		min	-76.485	3	-.544	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.096	3	0	1	0	3	0	1
564		min	-76.55	3	-.652	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.096	3	0	1	0	3	0	1
566		min	-76.615	3	-.761	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.096	3	0	1	0	3	0	1
568		min	-76.68	3	-.87	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.096	3	0	1	0	3	0	1
570		min	-76.745	3	-.978	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.674	4	.035	1	0	3	0	3	0	1
572		min	-75.637	3	0	2	-.039	3	0	1	0	1	0	1
573	2	max	0	2	1.488	4	.035	1	0	3	0	3	0	2
574		min	-75.572	3	0	2	-.039	3	0	1	0	1	0	4
575	3	max	0	2	1.302	4	.035	1	0	3	0	3	0	2
576		min	-75.506	3	0	2	-.039	3	0	1	0	1	0	4
577	4	max	0	2	1.116	4	.035	1	0	3	0	3	0	2
578		min	-75.441	3	0	2	-.039	3	0	1	0	1	-.001	4
579	5	max	0	2	.93	4	.035	1	0	3	0	3	0	2
580		min	-75.376	3	0	2	-.039	3	0	1	0	1	-.002	4
581	6	max	0	2	.744	4	.035	1	0	3	0	3	0	2
582		min	-75.311	3	0	2	-.039	3	0	1	0	1	-.002	4
583	7	max	0	2	.558	4	.035	1	0	3	0	3	0	2
584		min	-75.246	3	0	2	-.039	3	0	1	0	1	-.002	4
585	8	max	0	2	.372	4	.035	1	0	3	0	3	0	2
586		min	-75.18	3	0	2	-.039	3	0	1	0	1	-.002	4
587	9	max	0	2	.186	4	.035	1	0	3	0	3	0	2
588		min	-75.115	3	0	2	-.039	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.035	1	0	3	0	3	0	2
590		min	-75.05	3	0	1	-.039	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.035	1	0	3	0	3	0	2
592		min	-74.985	3	-.186	4	-.039	3	0	1	0	1	-.002	4
593	12	max	0	2	0	2	.035	1	0	3	0	3	0	2
594		min	-74.92	3	-.372	4	-.039	3	0	1	0	1	-.002	4
595	13	max	.072	13	0	2	.035	1	0	3	0	1	0	2
596		min	-74.855	3	-.558	4	-.039	3	0	1	0	4	-.002	4
597	14	max	.161	13	0	2	.035	1	0	3	0	1	0	2
598		min	-74.789	3	-.744	4	-.039	3	0	1	0	3	-.002	4
599	15	max	.251	13	0	2	.035	1	0	3	0	1	0	2
600		min	-74.724	3	-.93	4	-.039	3	0	1	0	3	-.002	4
601	16	max	.341	13	0	2	.035	1	0	3	0	1	0	2
602		min	-74.659	3	-1.116	4	-.039	3	0	1	0	3	-.001	4
603	17	max	.43	13	0	2	.035	1	0	3	0	1	0	2
604		min	-74.594	3	-1.302	4	-.039	3	0	1	0	3	0	4
605	18	max	.52	13	0	2	.035	1	0	3	0	1	0	2
606		min	-74.529	3	-1.488	4	-.039	3	0	1	0	3	0	4
607	19	max	.61	13	0	2	.035	1	0	3	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-74.463	3	-1.674	4	-.039	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.008	2	.005	1	-1.567e-5	15	NC	3	NC	2	
2			min	-.003	3	-.007	3	-.002	3	-4.131e-4	1	4693.72	2	7261.958	1	
3			2	max	.002	1	.007	2	.005	1	-1.501e-5	15	NC	3	NC	2
4				min	-.003	3	-.007	3	-.001	3	-3.955e-4	1	5117.842	2	7841.324	1
5			3	max	.002	1	.006	2	.004	1	-1.435e-5	15	NC	1	NC	2
6				min	-.003	3	-.007	3	-.001	3	-3.779e-4	1	5621.511	2	8524.922	1
7			4	max	.002	1	.006	2	.004	1	-1.369e-5	15	NC	1	NC	2
8				min	-.003	3	-.007	3	-.001	3	-3.603e-4	1	6223.938	2	9337.92	1
9			5	max	.002	1	.005	2	.004	1	-1.303e-5	15	NC	1	NC	1
10				min	-.002	3	-.006	3	-.001	3	-3.427e-4	1	6950.743	2	NC	1
11		6	max	.002	1	.005	2	.003	1	-1.237e-5	15	NC	1	NC	1	
12			min	-.002	3	-.006	3	0	3	-3.25e-4	1	7836.66	2	NC	1	
13		7	max	.001	1	.004	2	.003	1	-1.171e-5	15	NC	1	NC	1	
14			min	-.002	3	-.006	3	0	3	-3.074e-4	1	8929.702	2	NC	1	
15		8	max	.001	1	.004	2	.002	1	-1.104e-5	15	NC	1	NC	1	
16			min	-.002	3	-.005	3	0	3	-2.898e-4	1	NC	1	NC	1	
17		9	max	.001	1	.003	2	.002	1	-1.038e-5	15	NC	1	NC	1	
18			min	-.002	3	-.005	3	0	3	-2.722e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.002	1	-9.723e-6	15	NC	1	NC	1	
20			min	-.002	3	-.005	3	0	3	-2.546e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	.002	1	-9.062e-6	15	NC	1	NC	1	
22			min	-.001	3	-.004	3	0	3	-2.369e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.001	1	-8.402e-6	15	NC	1	NC	1	
24			min	-.001	3	-.004	3	0	3	-2.193e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	-7.741e-6	15	NC	1	NC	1	
26			min	-.001	3	-.003	3	0	3	-2.017e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	-7.08e-6	15	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-1.841e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-6.419e-6	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-1.665e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-5.692e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-1.489e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-4.827e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-1.312e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-3.963e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-1.136e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-3.098e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-9.599e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	4.469e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	1.449e-6	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	5.673e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	2.123e-6	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	6.877e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	2.646e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	8.081e-5	1	NC	1	NC	1
46				min	0	2	-.002	3	0	1	3.091e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	9.285e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	3.535e-6	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	1.049e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	0	1	3.979e-6	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	1.169e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.004	3	0	1	4.424e-6	15	NC	1	NC	1
53		8	max	0	3	0	2	0	3	1.29e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	4.868e-6	15	NC	1	NC	1
55		9	max	0	3	.001	2	0	2	1.41e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	5.313e-6	15	NC	1	NC	1
57		10	max	0	3	.001	2	0	1	1.53e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	15	5.757e-6	15	NC	1	NC	1
59		11	max	0	3	.002	2	0	1	1.651e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	15	6.202e-6	15	NC	1	NC	1
61		12	max	0	3	.002	2	0	1	1.771e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	15	6.646e-6	15	NC	1	NC	1
63		13	max	0	3	.003	2	.001	1	1.892e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	15	7.091e-6	15	NC	1	NC	1
65		14	max	.001	3	.004	2	.001	1	2.012e-4	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	15	7.535e-6	15	NC	1	NC	1
67		15	max	.001	3	.005	2	.002	1	2.132e-4	1	NC	1	NC	1
68			min	-.001	2	-.007	3	0	15	7.98e-6	15	NC	1	NC	1
69		16	max	.001	3	.005	2	.002	1	2.253e-4	1	NC	1	NC	1
70			min	-.001	2	-.007	3	0	15	8.424e-6	15	8466.723	2	NC	1
71		17	max	.001	3	.006	2	.002	1	2.373e-4	1	NC	1	NC	1
72			min	-.001	2	-.007	3	0	15	8.869e-6	15	7230.115	2	NC	1
73		18	max	.001	3	.007	2	.003	1	2.494e-4	1	NC	3	NC	1
74			min	-.001	2	-.007	3	0	15	9.313e-6	15	6273.957	2	NC	1
75		19	max	.001	3	.008	2	.003	1	2.614e-4	1	NC	3	NC	1
76			min	-.001	2	-.007	3	0	15	9.757e-6	15	5526.807	2	NC	1
77	M4	1	max	.002	1	.009	2	0	15	-1.284e-5	15	NC	1	NC	2
78			min	0	3	-.007	3	-.002	1	-3.366e-4	1	NC	1	8900.261	1
79		2	max	.001	1	.008	2	0	15	-1.284e-5	15	NC	1	NC	2
80			min	0	3	-.007	3	-.002	1	-3.366e-4	1	NC	1	9709.783	1
81		3	max	.001	1	.008	2	0	15	-1.284e-5	15	NC	1	NC	1
82			min	0	3	-.007	3	-.002	1	-3.366e-4	1	NC	1	NC	1
83		4	max	.001	1	.007	2	0	15	-1.284e-5	15	NC	1	NC	1
84			min	0	3	-.006	3	-.002	1	-3.366e-4	1	NC	1	NC	1
85		5	max	.001	1	.007	2	0	15	-1.284e-5	15	NC	1	NC	1
86			min	0	3	-.006	3	-.001	1	-3.366e-4	1	NC	1	NC	1
87		6	max	.001	1	.006	2	0	15	-1.284e-5	15	NC	1	NC	1
88			min	0	3	-.005	3	-.001	1	-3.366e-4	1	NC	1	NC	1
89		7	max	.001	1	.006	2	0	15	-1.284e-5	15	NC	1	NC	1
90			min	0	3	-.005	3	-.001	1	-3.366e-4	1	NC	1	NC	1
91		8	max	0	1	.005	2	0	15	-1.284e-5	15	NC	1	NC	1
92			min	0	3	-.005	3	0	1	-3.366e-4	1	NC	1	NC	1
93		9	max	0	1	.005	2	0	15	-1.284e-5	15	NC	1	NC	1
94			min	0	3	-.004	3	0	1	-3.366e-4	1	NC	1	NC	1
95		10	max	0	1	.004	2	0	15	-1.284e-5	15	NC	1	NC	1
96			min	0	3	-.004	3	0	1	-3.366e-4	1	NC	1	NC	1
97		11	max	0	1	.004	2	0	15	-1.284e-5	15	NC	1	NC	1
98			min	0	3	-.003	3	0	1	-3.366e-4	1	NC	1	NC	1
99		12	max	0	1	.003	2	0	15	-1.284e-5	15	NC	1	NC	1
100			min	0	3	-.003	3	0	1	-3.366e-4	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	15	-1.284e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-3.366e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-1.284e-5	15	NC	1	NC	1
104			min	0	3	-.002	3	0	1	-3.366e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-1.284e-5	15	NC	1	NC	1
106			min	0	3	-.002	3	0	1	-3.366e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-1.284e-5	15	NC	1	NC	1
108			min	0	3	-.001	3	0	1	-3.366e-4	1	NC	1	NC	1





Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-8.975e-8	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.106e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-8.975e-8	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-2.106e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-8.975e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.106e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.008	2	0	3	4.227e-4	1	NC	3	NC	1
230			min	-.003	3	-.007	3	-.001	1	-4.367e-4	3	4700.076	2	NC	1
231		2	max	.002	1	.007	2	0	3	4.014e-4	1	NC	3	NC	1
232			min	-.003	3	-.007	3	-.001	1	-4.226e-4	3	5124.951	2	NC	1
233		3	max	.002	1	.006	2	0	3	3.801e-4	1	NC	1	NC	1
234			min	-.002	3	-.007	3	-.001	1	-4.086e-4	3	5629.548	2	NC	1
235		4	max	.002	1	.006	2	0	3	3.588e-4	1	NC	1	NC	1
236			min	-.002	3	-.007	3	-.001	1	-3.946e-4	3	6233.13	2	NC	1
237		5	max	.002	1	.005	2	0	3	3.375e-4	1	NC	1	NC	1
238			min	-.002	3	-.006	3	-.001	1	-3.805e-4	3	6961.388	2	NC	1
239		6	max	.002	1	.005	2	0	3	3.162e-4	1	NC	1	NC	1
240			min	-.002	3	-.006	3	0	1	-3.665e-4	3	7849.154	2	NC	1
241		7	max	.001	1	.004	2	0	3	2.949e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	0	1	-3.525e-4	3	8944.584	2	NC	1
243		8	max	.001	1	.004	2	0	3	2.736e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	0	1	-3.385e-4	3	NC	1	NC	1
245		9	max	.001	1	.003	2	0	3	2.522e-4	1	NC	1	NC	1
246			min	-.002	3	-.005	3	0	1	-3.244e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	2.309e-4	1	NC	1	NC	1
248			min	-.001	3	-.005	3	0	1	-3.104e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	2.096e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-2.964e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	1.883e-4	1	NC	1	NC	1
252			min	-.001	3	-.004	3	0	1	-2.824e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	1.67e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-2.683e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.457e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-2.543e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.244e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.403e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.031e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.262e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	8.179e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.122e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	6.048e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.982e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.918e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.842e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.581e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.859e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	6.588e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-3.608e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	4.594e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.357e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	2.601e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-7.106e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	6.069e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-8.855e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	2	-4.054e-6	15	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-1.06e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-4.739e-6	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-1.235e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	-5.423e-6	15	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-1.41e-4	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	-6.107e-6	15	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-1.585e-4	1	NC	1	NC	1
285		10	max	0	3	.001	2	0	10	-6.792e-6	15	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-1.76e-4	1	NC	1	NC	1
287		11	max	0	3	.002	2	0	10	-7.476e-6	15	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-1.935e-4	1	NC	1	NC	1
289		12	max	0	3	.002	2	0	10	-8.16e-6	15	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-2.11e-4	1	NC	1	NC	1
291		13	max	0	3	.003	2	0	10	-8.845e-6	15	NC	1	NC	1
292			min	0	2	-.007	3	-.003	1	-2.285e-4	1	NC	1	NC	1
293		14	max	.001	3	.004	2	0	10	-9.529e-6	15	NC	1	NC	1
294			min	-.001	2	-.007	3	-.003	1	-2.459e-4	1	NC	1	NC	1
295		15	max	.001	3	.005	2	0	10	-1.021e-5	15	NC	1	NC	1
296			min	-.001	2	-.007	3	-.003	1	-2.634e-4	1	NC	1	NC	1
297		16	max	.001	3	.005	2	0	10	-1.09e-5	15	NC	1	NC	1
298			min	-.001	2	-.008	3	-.004	1	-2.809e-4	1	8478.975	2	NC	1
299		17	max	.001	3	.006	2	0	10	-1.158e-5	15	NC	1	NC	1
300			min	-.001	2	-.008	3	-.004	1	-2.984e-4	1	7239.563	2	NC	1
301		18	max	.001	3	.007	2	0	10	-1.227e-5	15	NC	3	NC	1
302			min	-.001	2	-.008	3	-.005	1	-3.159e-4	1	6281.456	2	NC	1
303		19	max	.001	3	.008	2	0	10	-1.295e-5	15	NC	3	NC	2
304			min	-.001	2	-.007	3	-.005	1	-3.334e-4	1	5532.924	2	9306.688	1
305	M12	1	max	.002	1	.009	2	.004	1	3.037e-4	1	NC	1	NC	2
306			min	0	3	-.007	3	0	10	1.163e-5	15	NC	1	4635.665	1
307		2	max	.001	1	.008	2	.004	1	3.037e-4	1	NC	1	NC	2
308			min	0	3	-.007	3	0	10	1.163e-5	15	NC	1	5055.805	1
309		3	max	.001	1	.008	2	.003	1	3.037e-4	1	NC	1	NC	2
310			min	0	3	-.007	3	0	10	1.163e-5	15	NC	1	5555.901	1
311		4	max	.001	1	.007	2	.003	1	3.037e-4	1	NC	1	NC	2
312			min	0	3	-.006	3	0	10	1.163e-5	15	NC	1	6157.029	1
313		5	max	.001	1	.007	2	.003	1	3.037e-4	1	NC	1	NC	2
314			min	0	3	-.006	3	0	10	1.163e-5	15	NC	1	6887.905	1
315		6	max	.001	1	.006	2	.002	1	3.037e-4	1	NC	1	NC	2
316			min	0	3	-.005	3	0	10	1.163e-5	15	NC	1	7788.465	1
317		7	max	.001	1	.006	2	.002	1	3.037e-4	1	NC	1	NC	2
318			min	0	3	-.005	3	0	10	1.163e-5	15	NC	1	8915.599	1
319		8	max	0	1	.005	2	.002	1	3.037e-4	1	NC	1	NC	1
320			min	0	3	-.005	3	0	10	1.163e-5	15	NC	1	NC	1
321		9	max	0	1	.005	2	.002	1	3.037e-4	1	NC	1	NC	1
322			min	0	3	-.004	3	0	10	1.163e-5	15	NC	1	NC	1
323		10	max	0	1	.004	2	.001	1	3.037e-4	1	NC	1	NC	1
324			min	0	3	-.004	3	0	10	1.163e-5	15	NC	1	NC	1
325		11	max	0	1	.004	2	.001	1	3.037e-4	1	NC	1	NC	1
326			min	0	3	-.003	3	0	10	1.163e-5	15	NC	1	NC	1
327		12	max	0	1	.003	2	0	1	3.037e-4	1	NC	1	NC	1
328			min	0	3	-.003	3	0	10	1.163e-5	15	NC	1	NC	1
329		13	max	0	1	.003	2	0	1	3.037e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	10	1.163e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	0	1	3.037e-4	1	NC	1	NC	1
332			min	0	3	-.002	3	0	10	1.163e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.037e-4	1	NC	1	NC	1
334			min	0	3	-.002	3	0	10	1.163e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	3.037e-4	1	NC	1	NC	1
336			min	0	3	-.001	3	0	10	1.163e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	3.037e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	10	1.163e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.037e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	10	1.163e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.037e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.163e-5	15	NC	1	NC	1
343	M1	1	max	.007	3	.023	3	.003	3	8.129e-3	1	NC	1	NC	1
344			min	-.007	2	-.02	2	-.002	1	-1.106e-2	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.002	3	3.912e-3	2	NC	4	NC	1
346			min	-.007	2	-.011	2	-.004	1	-5.456e-3	3	5020.156	3	NC	1
347		3	max	.007	3	.004	3	.002	3	4.554e-5	3	NC	4	NC	1
348			min	-.007	2	-.003	2	-.005	1	-2.371e-4	1	2603.984	3	NC	1
349		4	max	.007	3	.004	2	.001	3	4.607e-5	3	NC	4	NC	1
350			min	-.007	2	-.003	3	-.006	1	-1.998e-4	1	1860.605	3	NC	1
351		5	max	.007	3	.01	2	0	3	4.661e-5	3	NC	4	NC	1
352			min	-.007	2	-.01	3	-.006	1	-1.624e-4	1	1494.122	2	NC	1
353		6	max	.007	3	.016	2	0	3	4.714e-5	3	NC	4	NC	1
354			min	-.007	2	-.015	3	-.005	1	-1.251e-4	1	1271.411	2	NC	1
355		7	max	.007	3	.02	2	0	3	4.768e-5	3	NC	5	NC	1
356			min	-.007	2	-.018	3	-.005	1	-8.775e-5	1	1134.285	2	NC	1
357		8	max	.007	3	.023	2	0	3	4.821e-5	3	NC	5	NC	1
358			min	-.007	2	-.021	3	-.004	1	-5.042e-5	1	1048.538	2	NC	1
359		9	max	.007	3	.025	2	0	3	4.874e-5	3	NC	5	NC	1
360			min	-.007	2	-.022	3	-.003	1	-1.791e-5	9	998.036	2	NC	1
361		10	max	.007	3	.026	2	0	3	4.928e-5	3	NC	5	NC	1
362			min	-.007	2	-.023	3	-.001	1	6.471e-7	15	975.103	2	NC	1
363		11	max	.007	3	.025	2	0	3	6.159e-5	1	NC	5	NC	1
364			min	-.007	2	-.022	3	0	9	2.09e-6	15	977.007	2	NC	1
365		12	max	.007	3	.024	2	0	1	9.893e-5	1	NC	5	NC	1
366			min	-.007	2	-.02	3	0	15	3.534e-6	15	1004.93	2	NC	1
367		13	max	.007	3	.021	2	.002	1	1.363e-4	1	NC	4	NC	1
368			min	-.007	2	-.017	3	0	15	4.977e-6	15	1064.598	2	NC	1
369		14	max	.007	3	.016	2	.003	1	1.736e-4	1	NC	4	NC	1
370			min	-.007	2	-.013	3	0	15	6.42e-6	15	1169.157	2	NC	1
371		15	max	.007	3	.011	2	.003	1	2.109e-4	1	NC	4	NC	1
372			min	-.007	2	-.009	3	0	15	7.864e-6	15	1347.404	2	NC	1
373		16	max	.007	3	.004	2	.003	1	2.378e-4	1	NC	4	NC	1
374			min	-.007	2	-.003	3	0	15	8.901e-6	15	1669.581	2	NC	1
375		17	max	.007	3	.003	3	.002	1	4.65e-5	3	NC	4	NC	1
376			min	-.007	2	-.005	2	0	15	-7.413e-7	9	2358.898	2	NC	1
377		18	max	.007	3	.01	3	0	1	5.473e-3	2	NC	4	NC	1
378			min	-.007	2	-.015	2	0	15	-2.601e-3	3	4567.043	2	NC	1
379		19	max	.007	3	.018	3	0	3	1.102e-2	2	NC	1	NC	1
380			min	-.007	2	-.025	2	-.001	1	-5.297e-3	3	NC	1	NC	1
381	M5	1	max	.021	3	.072	3	.003	3	3.801e-6	3	NC	1	NC	1
382			min	-.025	2	-.064	2	-.002	1	0	15	NC	1	NC	1
383		2	max	.021	3	.041	3	.004	3	9.893e-5	3	NC	4	NC	1
384			min	-.025	2	-.036	2	-.002	1	-3.355e-5	1	1568.618	3	NC	1
385		3	max	.021	3	.013	3	.005	3	1.922e-4	3	NC	5	NC	1
386			min	-.025	2	-.009	2	-.002	1	-6.651e-5	1	813.969	3	NC	1
387		4	max	.021	3	.014	2	.006	3	1.873e-4	3	NC	5	NC	1
388			min	-.025	2	-.011	3	-.002	1	-6.299e-5	1	577.603	2	NC	1
389		5	max	.021	3	.034	2	.006	3	1.824e-4	3	NC	5	NC	1
390			min	-.025	2	-.031	3	-.002	1	-5.947e-5	1	457.694	2	NC	1
391		6	max	.021	3	.051	2	.006	3	1.775e-4	3	NC	5	NC	1
392			min	-.025	2	-.047	3	-.002	1	-5.595e-5	1	389.234	2	NC	1
393		7	max	.021	3	.065	2	.006	3	1.726e-4	3	NC	5	NC	1



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Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.025	2	-.058	3	-.002	1	-5.243e-5	1	347.063	2	NC	1
395		8	max	.021	3	.075	2	.006	3	1.677e-4	3	NC	5	NC	1
396			min	-.025	2	-.066	3	-.002	1	-4.891e-5	1	320.672	2	NC	1
397		9	max	.021	3	.081	2	.006	3	1.628e-4	3	NC	5	NC	1
398			min	-.025	2	-.071	3	-.001	1	-4.538e-5	1	305.099	2	NC	1
399		10	max	.021	3	.084	2	.006	3	1.579e-4	3	NC	5	NC	1
400			min	-.025	2	-.072	3	-.001	1	-4.186e-5	1	297.983	2	NC	1
401		11	max	.021	3	.083	2	.005	3	1.53e-4	3	NC	5	NC	1
402			min	-.025	2	-.069	3	-.001	1	-3.834e-5	1	298.48	2	NC	1
403		12	max	.021	3	.077	2	.005	3	1.481e-4	3	NC	5	NC	1
404			min	-.025	2	-.063	3	-.001	1	-3.482e-5	1	306.947	2	NC	1
405		13	max	.021	3	.068	2	.004	3	1.432e-4	3	NC	5	NC	1
406			min	-.025	2	-.055	3	-.001	1	-3.13e-5	1	325.13	2	NC	1
407		14	max	.021	3	.054	2	.004	3	1.383e-4	3	NC	5	NC	1
408			min	-.025	2	-.043	3	-.001	1	-2.778e-5	1	357.053	2	NC	1
409		15	max	.021	3	.035	2	.003	3	1.335e-4	3	NC	5	NC	1
410			min	-.025	2	-.028	3	-.001	1	-2.492e-5	9	411.533	2	NC	1
411		16	max	.021	3	.012	2	.002	3	1.247e-4	3	NC	5	NC	1
412			min	-.025	2	-.01	3	-.001	1	-2.381e-5	9	510.107	2	NC	1
413		17	max	.021	3	.01	3	.002	3	2.546e-5	3	NC	5	NC	1
414			min	-.025	2	-.016	2	0	1	-7.524e-5	1	721.502	2	NC	1
415		18	max	.021	3	.032	3	.001	3	1.162e-5	3	NC	4	NC	1
416			min	-.025	2	-.048	2	0	1	-3.849e-5	1	1397.687	2	NC	1
417		19	max	.021	3	.055	3	0	3	0	9	NC	1	NC	1
418			min	-.025	2	-.083	2	0	1	-6.179e-7	3	NC	1	NC	1
419	M9	1	max	.007	3	.022	3	.002	3	1.107e-2	3	NC	1	NC	1
420			min	-.007	2	-.02	2	-.002	1	-8.129e-3	1	NC	1	NC	1
421		2	max	.007	3	.013	3	.001	3	5.471e-3	3	NC	4	NC	1
422			min	-.007	2	-.011	2	0	9	-3.98e-3	1	5021.918	3	NC	1
423		3	max	.007	3	.004	3	.001	1	9.139e-5	1	NC	4	NC	1
424			min	-.007	2	-.003	2	0	3	-2.415e-5	3	2604.917	3	NC	1
425		4	max	.007	3	.004	2	.002	1	6.014e-5	1	NC	4	NC	1
426			min	-.007	2	-.004	3	-.001	3	-3.126e-5	3	1861.255	3	NC	1
427		5	max	.007	3	.01	2	.002	1	3.045e-5	2	NC	4	NC	1
428			min	-.007	2	-.01	3	-.002	3	-3.836e-5	3	1494.488	2	NC	1
429		6	max	.007	3	.016	2	.002	1	1.965e-5	2	NC	4	NC	1
430			min	-.007	2	-.015	3	-.003	3	-4.547e-5	3	1271.737	2	NC	1
431		7	max	.007	3	.02	2	.002	1	8.855e-6	2	NC	5	NC	1
432			min	-.007	2	-.019	3	-.003	3	-5.257e-5	3	1134.588	2	9330.935	3
433		8	max	.007	3	.023	2	0	1	-7.025e-7	10	NC	5	NC	1
434			min	-.007	2	-.021	3	-.004	3	-6.487e-5	1	1048.831	2	8832.005	3
435		9	max	.007	3	.025	2	0	2	-2.206e-6	10	NC	5	NC	1
436			min	-.007	2	-.022	3	-.004	3	-9.612e-5	1	998.325	2	8612.896	3
437		10	max	.007	3	.026	2	0	2	-3.71e-6	10	NC	5	NC	1
438			min	-.007	2	-.023	3	-.004	3	-1.274e-4	1	975.395	2	8624.443	3
439		11	max	.007	3	.025	2	0	10	-5.214e-6	10	NC	5	NC	1
440			min	-.007	2	-.022	3	-.004	3	-1.586e-4	1	977.308	2	8857.698	3
441		12	max	.007	3	.024	2	0	10	-6.717e-6	10	NC	5	NC	1
442			min	-.007	2	-.02	3	-.004	3	-1.899e-4	1	1005.248	2	9338.571	3
443		13	max	.007	3	.021	2	0	10	-8.221e-6	10	NC	4	NC	1
444			min	-.007	2	-.017	3	-.004	1	-2.211e-4	1	1064.942	2	NC	1
445		14	max	.007	3	.016	2	0	10	-9.725e-6	10	NC	4	NC	1
446			min	-.007	2	-.014	3	-.005	1	-2.524e-4	1	1169.542	2	NC	1
447		15	max	.007	3	.011	2	0	10	-1.098e-5	15	NC	4	NC	1
448			min	-.007	2	-.009	3	-.005	1	-2.836e-4	1	1347.85	2	NC	1
449		16	max	.007	3	.004	2	0	10	-1.191e-5	15	NC	4	NC	1
450			min	-.007	2	-.003	3	-.005	1	-3.079e-4	1	1670.128	2	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.007	3	.003	3	0	10	2.777e-5	3	NC	4	NC	1
452			min	-.007	2	-.005	2	-.004	1	-1.659e-4	1	2359.615	2	NC	1
453		18	max	.007	3	.01	3	0	10	2.639e-3	3	NC	4	NC	1
454			min	-.007	2	-.015	2	-.003	1	-5.474e-3	2	4568.387	2	NC	1
455		19	max	.007	3	.018	3	0	3	5.296e-3	3	NC	1	NC	1
456			min	-.007	2	-.025	2	0	1	-1.102e-2	2	NC	1	NC	1
457	M13	1	max	.002	1	.022	3	.007	3	3.626e-3	3	NC	1	NC	1
458			min	-.002	3	-.02	2	-.007	2	-3.277e-3	2	NC	1	NC	1
459		2	max	.002	1	.09	3	.006	9	4.518e-3	3	NC	4	NC	1
460			min	-.002	3	-.068	2	-.005	2	-4.098e-3	2	1682.812	3	NC	1
461		3	max	.002	1	.146	3	.017	1	5.409e-3	3	NC	5	NC	2
462			min	-.002	3	-.109	1	-.004	10	-4.919e-3	2	919.581	3	5530.929	1
463		4	max	.002	1	.183	3	.025	1	6.301e-3	3	NC	5	NC	2
464			min	-.002	3	-.137	1	-.004	10	-5.741e-3	2	708.642	3	3883.001	1
465		5	max	.002	1	.197	3	.028	1	7.192e-3	3	NC	5	NC	2
466			min	-.002	3	-.148	1	-.005	10	-6.562e-3	2	652.577	3	3510.267	1
467		6	max	.002	1	.188	3	.025	1	8.084e-3	3	NC	5	NC	2
468			min	-.002	3	-.142	2	-.007	10	-7.383e-3	2	687.876	3	3957.726	1
469		7	max	.002	1	.161	3	.016	9	8.975e-3	3	NC	5	NC	2
470			min	-.003	3	-.124	2	-.009	2	-8.204e-3	2	824.164	3	5996.356	1
471		8	max	.002	1	.123	3	.016	3	9.867e-3	3	NC	5	NC	1
472			min	-.003	3	-.099	2	-.016	2	-9.026e-3	2	1130.156	3	NC	1
473		9	max	.002	1	.088	3	.019	3	1.076e-2	3	NC	4	NC	1
474			min	-.003	3	-.075	2	-.022	2	-9.847e-3	2	1731.986	3	7634.805	2
475		10	max	.002	1	.072	3	.021	3	1.165e-2	3	NC	4	NC	4
476			min	-.003	3	-.064	2	-.025	2	-1.067e-2	2	2295.319	3	6456.484	2
477		11	max	.002	1	.088	3	.023	3	1.076e-2	3	NC	4	NC	1
478			min	-.003	3	-.075	2	-.022	2	-9.847e-3	2	1731.985	3	7031.349	3
479		12	max	.002	1	.123	3	.024	3	9.87e-3	3	NC	5	NC	1
480			min	-.003	3	-.099	2	-.016	2	-9.026e-3	2	1130.155	3	6748.83	3
481		13	max	.002	1	.161	3	.023	3	8.979e-3	3	NC	5	NC	2
482			min	-.003	3	-.124	2	-.009	2	-8.204e-3	2	824.163	3	5968.689	1
483		14	max	.002	1	.188	3	.025	1	8.089e-3	3	NC	5	NC	2
484			min	-.003	3	-.142	2	-.007	10	-7.383e-3	2	687.876	3	3953.785	1
485		15	max	.002	1	.197	3	.028	1	7.199e-3	3	NC	5	NC	2
486			min	-.003	3	-.148	1	-.005	10	-6.562e-3	2	652.576	3	3514.649	1
487		16	max	.002	1	.183	3	.025	1	6.309e-3	3	NC	5	NC	2
488			min	-.003	3	-.137	1	-.004	10	-5.741e-3	2	708.642	3	3896.103	1
489		17	max	.002	1	.146	3	.016	1	5.419e-3	3	NC	5	NC	2
490			min	-.003	3	-.109	1	-.004	10	-4.919e-3	2	919.58	3	5564.74	1
491		18	max	.002	1	.09	3	.009	3	4.528e-3	3	NC	4	NC	1
492			min	-.003	3	-.068	2	-.005	2	-4.098e-3	2	1682.811	3	NC	1
493		19	max	.002	1	.023	3	.007	3	3.638e-3	3	NC	1	NC	1
494			min	-.003	3	-.02	2	-.007	2	-3.277e-3	2	NC	1	NC	1
495	M16	1	max	0	1	.018	3	.007	3	4.004e-3	2	NC	1	NC	1
496			min	0	3	-.025	2	-.007	2	-2.8e-3	3	NC	1	NC	1
497		2	max	0	1	.05	3	.009	3	5.015e-3	2	NC	4	NC	1
498			min	0	3	-.093	2	-.005	2	-3.467e-3	3	1688.491	2	NC	1
499		3	max	0	1	.077	3	.016	1	6.025e-3	2	NC	5	NC	2
500			min	0	3	-.149	2	-.004	10	-4.135e-3	3	921.068	2	5544.866	1
501		4	max	0	1	.096	3	.025	1	7.035e-3	2	NC	5	NC	2
502			min	0	3	-.186	2	-.004	10	-4.802e-3	3	707.567	2	3893.3	1
503		5	max	0	1	.104	3	.028	1	8.046e-3	2	NC	5	NC	2
504			min	0	3	-.201	2	-.005	10	-5.469e-3	3	648.256	2	3521.438	1
505		6	max	0	1	.102	3	.024	1	9.056e-3	2	NC	5	NC	2
506			min	0	3	-.193	2	-.007	10	-6.137e-3	3	677.593	2	3975.326	1
507		7	max	0	1	.092	3	.022	3	1.007e-2	2	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-168	2	-.009	2	-6.804e-3	3	800.16	2	6044.306	1
509	8	max	0	1	.077	3	.022	3	1.108e-2	2	NC	5	NC	1
510		min	0	3	-.132	2	-.016	2	-7.472e-3	3	1068.535	2	7449.024	3
511	9	max	0	1	.062	3	.022	3	1.209e-2	2	NC	4	NC	1
512		min	0	3	-.098	2	-.022	2	-8.139e-3	3	1561.959	2	7630.698	3
513	10	max	0	1	.055	3	.021	3	1.31e-2	2	NC	4	NC	4
514		min	0	3	-.083	2	-.025	2	-8.806e-3	3	1985.948	2	6454.384	2
515	11	max	0	1	.062	3	.019	3	1.209e-2	2	NC	4	NC	1
516		min	0	3	-.098	2	-.022	2	-8.138e-3	3	1561.959	2	7631.062	2
517	12	max	0	1	.077	3	.018	3	1.108e-2	2	NC	5	NC	1
518		min	0	3	-.132	2	-.016	2	-7.469e-3	3	1068.535	2	9786.382	3
519	13	max	0	1	.092	3	.017	3	1.007e-2	2	NC	5	NC	2
520		min	0	3	-168	2	-.009	2	-6.801e-3	3	800.16	2	6036.831	1
521	14	max	0	1	.102	3	.024	1	9.057e-3	2	NC	5	NC	2
522		min	0	3	-.193	2	-.007	10	-6.132e-3	3	677.593	2	3981.322	1
523	15	max	.001	1	.104	3	.028	1	8.047e-3	2	NC	5	NC	2
524		min	0	3	-.201	2	-.005	10	-5.463e-3	3	648.256	2	3533.699	1
525	16	max	.001	1	.096	3	.025	1	7.036e-3	2	NC	5	NC	2
526		min	0	3	-.186	2	-.004	10	-4.795e-3	3	707.567	2	3915.137	1
527	17	max	.001	1	.077	3	.016	1	6.026e-3	2	NC	5	NC	2
528		min	0	3	-.149	2	-.004	10	-4.126e-3	3	921.068	2	5592.584	1
529	18	max	.001	1	.05	3	.008	3	5.016e-3	2	NC	4	NC	1
530		min	0	3	-.093	2	-.005	2	-3.457e-3	3	1688.491	2	NC	1
531	19	max	.001	1	.018	3	.007	3	4.006e-3	2	NC	1	NC	1
532		min	0	3	-.025	2	-.007	2	-2.789e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	3.625e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.173e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	8.01e-4	3	NC	1	NC	1
536		min	0	2	-.004	4	0	3	-4.957e-4	2	NC	1	NC	1
537	3	max	0	3	-.002	15	.003	1	1.24e-3	3	NC	1	NC	1
538		min	0	2	-.008	4	-.003	3	-9.397e-4	2	8209.481	4	NC	1
539	4	max	0	3	-.003	15	.006	1	1.678e-3	3	NC	5	NC	4
540		min	0	2	-.012	4	-.007	3	-1.384e-3	2	5632.183	4	6230.592	3
541	5	max	0	3	-.004	15	.01	1	2.117e-3	3	NC	5	NC	4
542		min	0	2	-.016	4	-.011	3	-1.828e-3	2	4394.85	4	4091.702	3
543	6	max	0	3	-.004	15	.014	1	2.555e-3	3	NC	5	NC	4
544		min	0	2	-.018	4	-.016	3	-2.272e-3	2	3698.731	4	2980.454	3
545	7	max	0	3	-.005	15	.018	1	2.994e-3	3	NC	15	NC	4
546		min	-.001	2	-.021	4	-.021	3	-2.716e-3	2	3280.107	4	2330.57	3
547	8	max	0	3	-.005	15	.022	1	3.432e-3	3	NC	15	NC	4
548		min	-.001	2	-.023	4	-.025	3	-3.16e-3	2	3028.869	4	1921.997	3
549	9	max	0	3	-.006	15	.026	1	3.871e-3	3	NC	15	NC	4
550		min	-.001	2	-.024	4	-.03	3	-3.604e-3	2	2893.639	4	1654.611	3
551	10	max	0	3	-.006	15	.029	1	4.309e-3	3	NC	15	NC	4
552		min	-.002	2	-.024	4	-.033	3	-4.048e-3	2	2850.858	4	1478.127	3
553	11	max	0	3	-.006	15	.031	1	4.748e-3	3	NC	15	NC	5
554		min	-.002	2	-.024	4	-.035	3	-4.491e-3	2	2893.639	4	1366.143	3
555	12	max	0	3	-.005	15	.032	1	5.186e-3	3	NC	15	NC	5
556		min	-.002	2	-.023	4	-.036	3	-4.935e-3	2	3028.869	4	1305.562	3
557	13	max	0	3	-.005	15	.031	1	5.625e-3	3	NC	15	NC	5
558		min	-.002	2	-.021	4	-.035	3	-5.379e-3	2	3280.107	4	1292.584	3
559	14	max	0	3	-.004	15	.029	1	6.063e-3	3	NC	5	NC	5
560		min	-.002	2	-.019	4	-.033	3	-5.823e-3	2	3698.731	4	1332.849	3
561	15	max	.001	3	-.004	15	.024	1	6.502e-3	3	NC	5	NC	4
562		min	-.003	2	-.016	4	-.027	3	-6.267e-3	2	4394.85	4	1447.064	3
563	16	max	.001	3	-.003	15	.017	1	6.94e-3	3	NC	5	NC	4
564		min	-.003	2	-.013	4	-.019	3	-6.711e-3	2	5632.183	4	1691.464	3

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.001	3	-.002	12	.008	1	7.379e-3	3	NC	1	NC	4
566			min	-.003	2	-.009	4	-.008	3	-7.155e-3	2	8209.481	4	2242.48	3
567		18	max	.001	3	0	2	.007	3	7.817e-3	3	NC	1	NC	4
568			min	-.003	2	-.005	4	-.01	2	-7.599e-3	2	NC	1	3992.619	3
569		19	max	.001	3	.004	2	.025	3	8.256e-3	3	NC	1	NC	1
570			min	-.003	2	-.002	9	-.026	2	-8.043e-3	2	NC	1	NC	1
571	M16A	1	max	0	10	0	10	.008	3	2.399e-3	3	NC	1	NC	1
572			min	-.001	3	0	9	-.008	2	-2.395e-3	2	NC	1	NC	1
573		2	max	0	10	-.001	15	.002	9	2.303e-3	3	NC	1	NC	1
574			min	-.001	3	-.005	4	-.003	2	-2.286e-3	2	NC	1	NC	1
575		3	max	0	10	-.002	15	.005	1	2.207e-3	3	NC	1	NC	4
576			min	-.001	3	-.009	4	-.004	3	-2.177e-3	2	8209.481	4	6371.308	3
577		4	max	0	10	-.003	15	.008	1	2.111e-3	3	NC	5	NC	4
578			min	-.001	3	-.012	4	-.007	3	-2.069e-3	2	5632.183	4	4847.76	3
579		5	max	0	10	-.004	15	.011	1	2.015e-3	3	NC	5	NC	4
580			min	-.001	3	-.016	4	-.01	3	-1.96e-3	2	4394.85	4	4188.636	3
581		6	max	0	10	-.004	15	.012	1	1.919e-3	3	NC	5	NC	4
582			min	0	3	-.019	4	-.012	3	-1.851e-3	2	3698.731	4	3902.295	3
583		7	max	0	10	-.005	15	.013	1	1.823e-3	3	NC	15	NC	4
584			min	0	3	-.021	4	-.012	3	-1.742e-3	2	3280.107	4	3834.941	3
585		8	max	0	10	-.005	15	.012	1	1.727e-3	3	NC	15	NC	4
586			min	0	3	-.023	4	-.012	3	-1.634e-3	2	3028.869	4	3934.401	3
587		9	max	0	10	-.006	15	.012	1	1.632e-3	3	NC	15	NC	4
588			min	0	3	-.024	4	-.012	3	-1.525e-3	2	2893.639	4	4194.484	3
589		10	max	0	10	-.006	15	.011	1	1.536e-3	3	NC	15	NC	4
590			min	0	3	-.024	4	-.011	3	-1.416e-3	2	2850.858	4	4642.417	3
591		11	max	0	10	-.006	15	.009	1	1.44e-3	3	NC	15	NC	4
592			min	0	3	-.024	4	-.009	3	-1.307e-3	2	2893.639	4	5345.219	3
593		12	max	0	10	-.005	15	.008	1	1.344e-3	3	NC	15	NC	4
594			min	0	3	-.023	4	-.007	3	-1.199e-3	2	3028.869	4	6436.366	3
595		13	max	0	10	-.005	15	.006	1	1.248e-3	3	NC	15	NC	2
596			min	0	3	-.021	4	-.006	3	-1.09e-3	2	3280.107	4	8184.509	3
597		14	max	0	10	-.004	15	.004	1	1.152e-3	3	NC	5	NC	1
598			min	0	3	-.018	4	-.004	3	-9.812e-4	2	3698.731	4	NC	1
599		15	max	0	10	-.004	15	.003	1	1.056e-3	3	NC	5	NC	1
600			min	0	3	-.015	4	-.002	3	-8.725e-4	2	4394.85	4	NC	1
601		16	max	0	10	-.003	15	.001	9	9.605e-4	3	NC	5	NC	1
602			min	0	3	-.012	4	0	3	-7.638e-4	2	5632.183	4	NC	1
603		17	max	0	10	-.002	15	0	4	8.646e-4	3	NC	1	NC	1
604			min	0	3	-.008	4	0	2	-6.55e-4	2	8209.481	4	NC	1
605		18	max	0	10	0	15	0	4	7.687e-4	3	NC	1	NC	1
606			min	0	3	-.004	4	0	2	-5.463e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	6.728e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-4.376e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

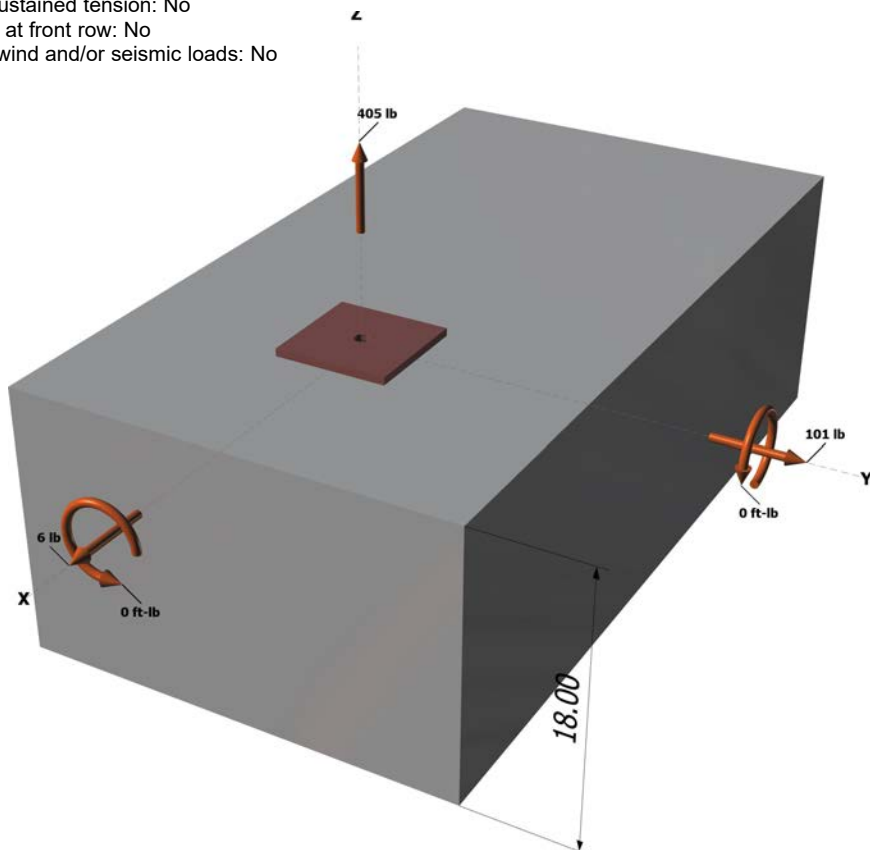
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

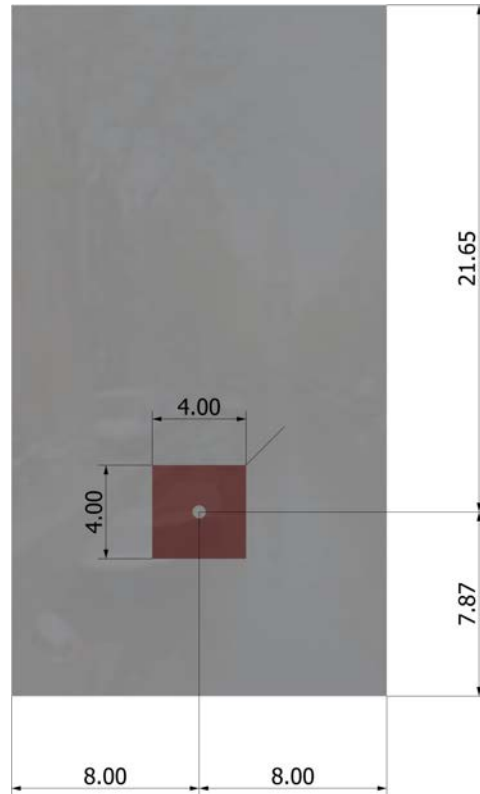
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

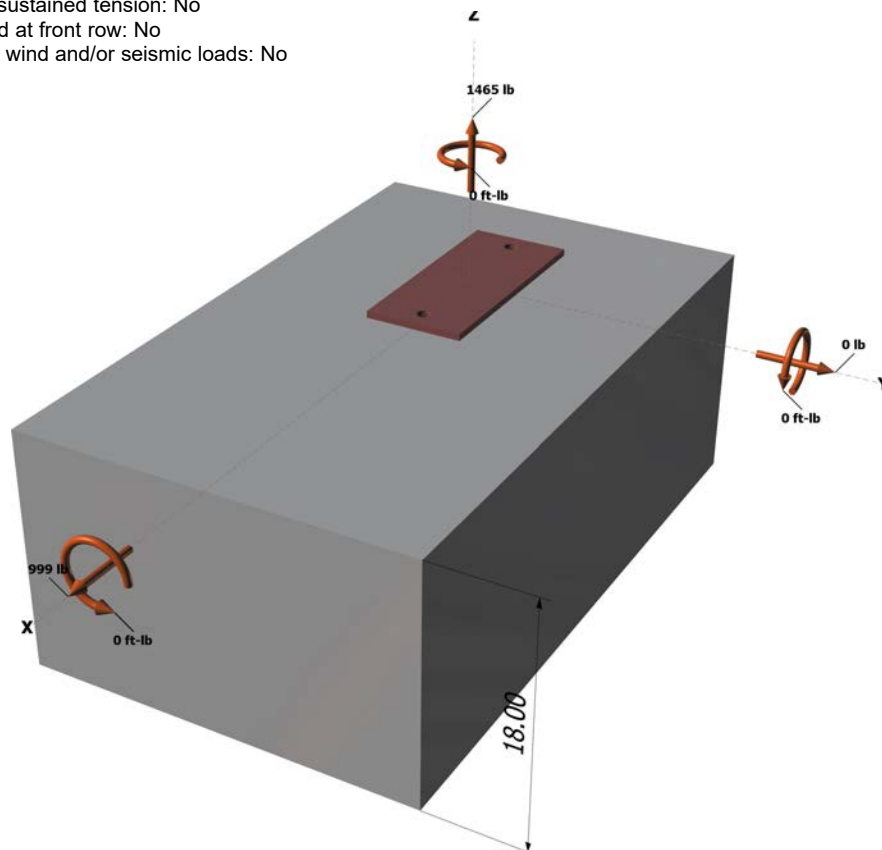
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



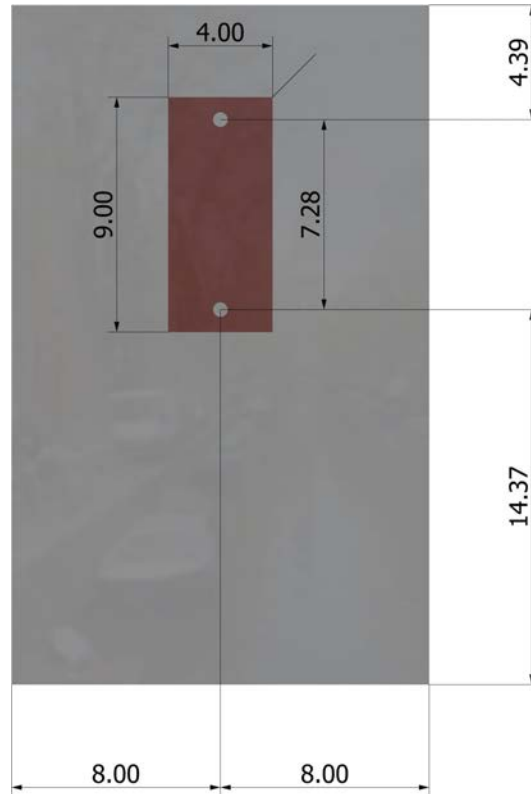
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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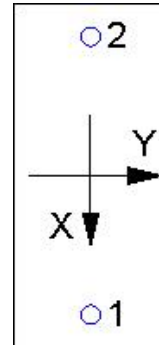
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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