

Ú&Q^α:ÊQ&È

PÔX

Úæ åæåÁÚXT å æÚæ å * Á^•c{
 Ü^] |^•^} ææ^ Ôæ& |ææ } • ÁÚÔÒÁ È€

Í »ÁæÁ ðÁ^ã{ æÔ^ã}

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

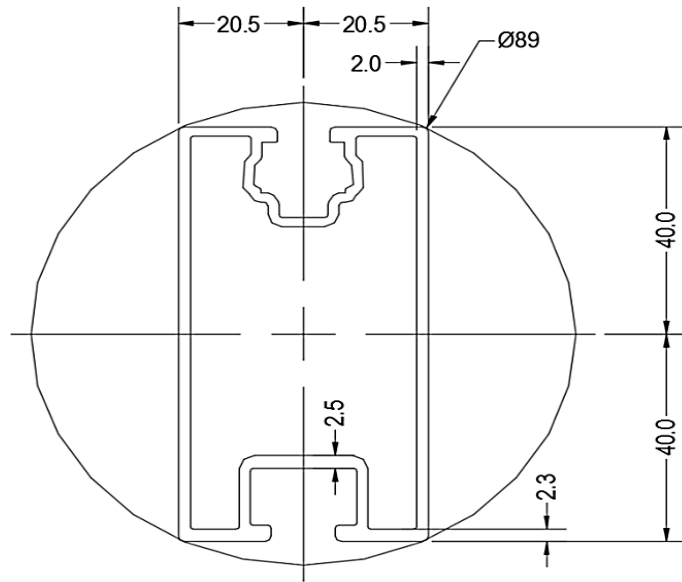
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

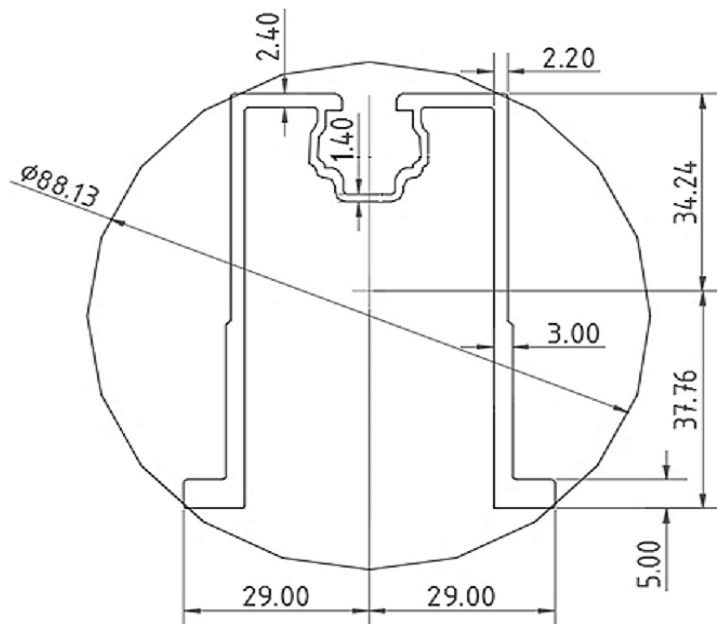
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	69 in
ΦF_{ty} STRONG-AXIS =	29.48 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.581 k-ft
M_z =	0.127 k-ft
$M_{y \text{ allowable}}$ =	1.831 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	47%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

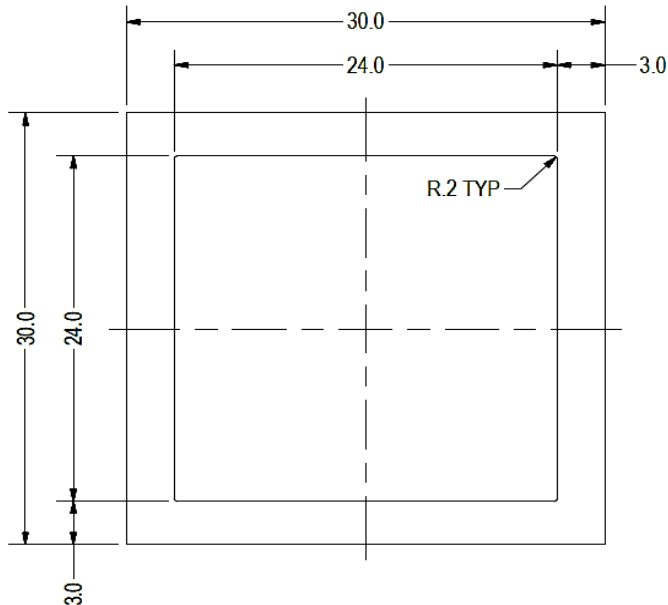
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.45 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.521 k-ft
M_z =	0.000 k-ft
P_n =	0.292 k
$M_{y \text{ allowable}}$ =	1.445 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.081 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

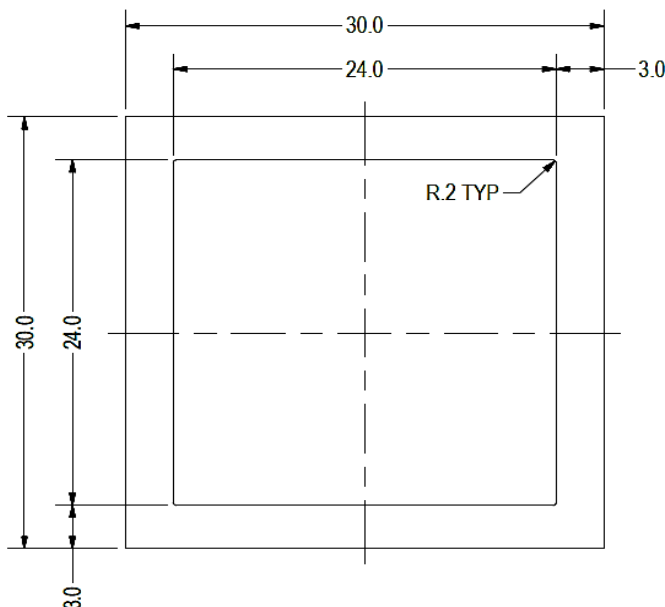
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.475 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.882 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.063 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	7%



A cross brace kit is required every 28 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

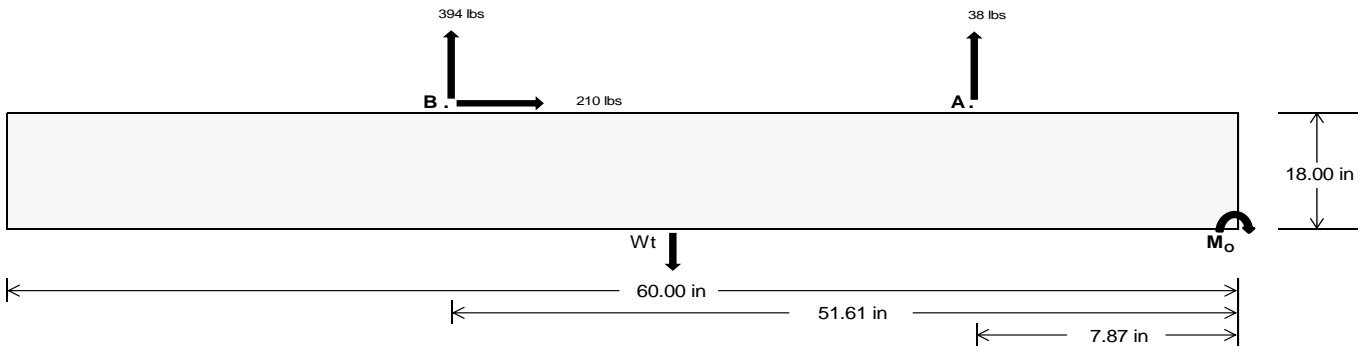
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	169.65	1711.20	k
Compressive Load =	1405.74	1225.47	k
Lateral Load =	2.57	911.42	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24406.0$ in-lbs
Resisting Force Required = 813.53 lbs
S.F. = 1.67
Weight Required = 1355.89 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 210.22 lbs
Friction = 0.4
Weight Required = 525.55 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 210.22 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	504 lbs	504 lbs	504 lbs	504 lbs	453 lbs	453 lbs	453 lbs	453 lbs	676 lbs	676 lbs	676 lbs	676 lbs	-75 lbs	-75 lbs	-75 lbs	-75 lbs
F_B	357 lbs	357 lbs	357 lbs	357 lbs	494 lbs	494 lbs	494 lbs	494 lbs	608 lbs	608 lbs	608 lbs	608 lbs	-788 lbs	-788 lbs	-788 lbs	-788 lbs
F_V	47 lbs	47 lbs	47 lbs	47 lbs	378 lbs	378 lbs	378 lbs	378 lbs	315 lbs	315 lbs	315 lbs	315 lbs	-420 lbs	-420 lbs	-420 lbs	-420 lbs
P_{total}	2765 lbs	2855 lbs	2946 lbs	3037 lbs	2850 lbs	2940 lbs	3031 lbs	3121 lbs	3187 lbs	3278 lbs	3368 lbs	3459 lbs	279 lbs	334 lbs	388 lbs	442 lbs
M	356 lbs-ft	356 lbs-ft	356 lbs-ft	356 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	625 lbs-ft	625 lbs-ft	625 lbs-ft	625 lbs-ft	650 lbs-ft	650 lbs-ft	650 lbs-ft	650 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.33 ft	1.95 ft	1.67 ft	1.47 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	267.1 psf	264.8 psf	262.8 psf	260.9 psf	255.4 psf	253.7 psf	252.1 psf	250.7 psf	278.6 psf	275.8 psf	273.3 psf	270.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	364.9 psf	358.2 psf	352.0 psf	346.4 psf	395.9 psf	387.8 psf	380.4 psf	373.6 psf	449.9 psf	439.3 psf	429.7 psf	420.8 psf	614.3 psf	219.6 psf	163.5 psf	143.0 psf

Maximum Bearing Pressure = 614 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

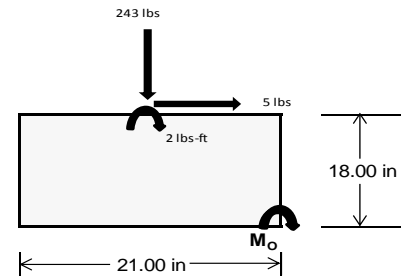
Overturning Check

$M_o = 203.7 \text{ ft-lbs}$
 Resisting Force Required = 232.81 lbs
 S.F. = 1.67
 Weight Required = 388.02 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	69 lbs	179 lbs	65 lbs	243 lbs	708 lbs	239 lbs	20 lbs	52 lbs	19 lbs
F_v	1 lbs	1 lbs	0 lbs	5 lbs	4 lbs	1 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2425 lbs	2535 lbs	2421 lbs	2486 lbs	2951 lbs	2482 lbs	709 lbs	741 lbs	708 lbs
M	1 lbs-ft	1 lbs-ft	0 lbs-ft	9 lbs-ft	6 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.74 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	276.7 sqft	289.2 sqft	276.7 sqft	280.6 sqft	334.8 sqft	283.4 sqft	80.9 sqft	84.6 sqft	80.9 sqft
f_{max}	277.7 psf	290.1 psf	276.8 psf	287.7 psf	339.7 psf	284.0 psf	81.2 psf	84.8 psf	80.9 psf



Maximum Bearing Pressure = 340 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

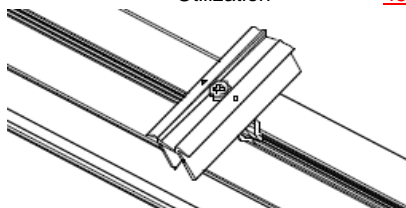
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

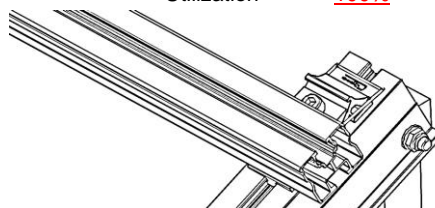
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.546 k
Allowable Uplift =	1.214 k
Utilization =	<u>45%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.113 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

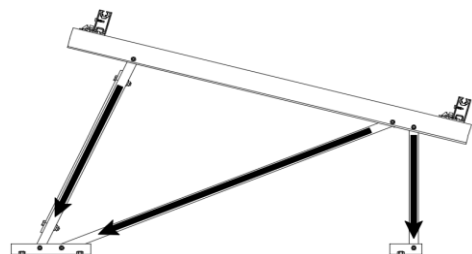
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.081 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Diagonal Strut

Maximum Axial Load =	0.475 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.170 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.063 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.017 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.427$$

$$143.909$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.427$$

$$156.378$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.3$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.95$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 38.1$$

$$m = 0.63$$

$$C_0 = 40.784$$

$$Cc = 39.216$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 79.7$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 498305 \text{ mm}^4$$

$$1.197 \text{ in}^4$$

$$y = 40.784 \text{ mm}$$

$$S_x = 0.746 \text{ in}^3$$

$$M_{\max} St = 1.831 \text{ k-ft}$$

3.4.18

$$h/t = 6.6$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20.5$$

$$Cc = 20.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 22.7 \text{ ksi}$$

$$I_y = 148662 \text{ mm}^4$$

$$0.357 \text{ in}^4$$

$$x = 20.5 \text{ mm}$$

$$S_y = 0.443 \text{ in}^3$$

$$M_{\max} Wk = 0.838 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 6.6$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

$$b/t = 37.95$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = (\phi k_2 \sqrt{(BpE)}) / (1.6b/t)$$

$$\phi F_L = 21.4 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 21.42 \text{ ksi}$$

$$A = 620.02 \text{ mm}^2$$

$$0.96 \text{ in}^2$$

$$P_{\max} = 20.59 \text{ kips}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1669 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.445 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	181.179	2	284.623	2	0	5	0	1	0	1	0	1
2		min	-224.671	3	-408.436	3	-.119	3	0	3	0	1	0	1
3	N7	max	0	15	389.294	1	-.035	15	0	15	0	1	0	1
4		min	-.161	2	-30.117	3	-.836	1	-.001	1	0	1	0	1
5	N15	max	0	15	1081.335	1	.49	1	0	1	0	1	0	1
6		min	-1.432	2	-130.501	3	-.443	3	0	3	0	1	0	1
7	N16	max	646.198	2	942.671	2	0	10	0	1	0	1	0	1
8		min	-701.09	3	-1316.31	3	-55.137	3	0	3	0	1	0	1
9	N23	max	0	15	389.099	1	1.974	1	.003	1	0	1	0	1
10		min	-.161	2	-29.639	3	.079	15	0	15	0	1	0	1
11	N24	max	181.358	2	287.893	2	55.56	3	0	1	0	1	0	1
12		min	-224.889	3	-406.577	3	.008	10	0	3	0	1	0	1
13	Totals:	max	1006.981	2	3287.323	1	0	9						
14		min	-1150.83	3	-2321.581	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	275.744	1	.643	4	.36	1	0	15	0	15	0	1
2			min	-369.152	3	.152	15	-.059	3	0	1	0	1	0	1
3		2	max	275.86	1	.597	4	.36	1	0	15	0	15	0	15
4			min	-369.065	3	.141	15	-.059	3	0	1	0	1	0	4
5		3	max	275.976	1	.552	4	.36	1	0	15	0	1	0	15
6			min	-368.978	3	.13	15	-.059	3	0	1	0	3	0	4
7		4	max	276.093	1	.506	4	.36	1	0	15	0	1	0	15
8			min	-368.89	3	.119	15	-.059	3	0	1	0	3	0	4
9		5	max	276.209	1	.46	4	.36	1	0	15	0	1	0	15
10			min	-368.803	3	.109	15	-.059	3	0	1	0	3	0	4
11		6	max	276.326	1	.415	4	.36	1	0	15	0	1	0	15
12			min	-368.716	3	.098	15	-.059	3	0	1	0	3	0	4
13		7	max	276.442	1	.369	4	.36	1	0	15	0	1	0	15
14			min	-368.629	3	.087	15	-.059	3	0	1	0	3	0	4
15		8	max	276.558	1	.323	4	.36	1	0	15	0	1	0	15
16			min	-368.541	3	.076	15	-.059	3	0	1	0	3	0	4
17		9	max	276.675	1	.278	4	.36	1	0	15	0	1	0	15
18			min	-368.454	3	.066	15	-.059	3	0	1	0	3	0	4
19		10	max	276.791	1	.232	4	.36	1	0	15	0	1	0	15
20			min	-368.367	3	.055	15	-.059	3	0	1	0	3	0	4
21		11	max	276.908	1	.186	4	.36	1	0	15	0	1	0	15
22			min	-368.279	3	.044	15	-.059	3	0	1	0	3	0	4
23		12	max	277.024	1	.141	4	.36	1	0	15	0	1	0	15
24			min	-368.192	3	.034	15	-.059	3	0	1	0	3	0	4
25		13	max	277.14	1	.102	2	.36	1	0	15	0	1	0	15
26			min	-368.105	3	.018	12	-.059	3	0	1	0	3	0	4
27		14	max	277.257	1	.067	2	.36	1	0	15	0	1	0	15
28			min	-368.017	3	-.003	3	-.059	3	0	1	0	3	0	4
29		15	max	277.373	1	.031	2	.36	1	0	15	0	1	0	15
30			min	-367.93	3	-.03	3	-.059	3	0	1	0	3	0	4
31		16	max	277.49	1	-.004	2	.36	1	0	15	0	1	0	15
32			min	-367.843	3	-.056	3	-.059	3	0	1	0	3	0	4
33		17	max	277.606	1	-.02	15	.36	1	0	15	0	1	0	15
34			min	-367.756	3	-.088	4	-.059	3	0	1	0	3	0	4
35		18	max	277.722	1	-.031	15	.36	1	0	15	0	1	0	15
36			min	-367.668	3	-.133	4	-.059	3	0	1	0	3	0	4
37		19	max	277.839	1	-.042	15	.36	1	0	15	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-367.581	3	-.179	4	-.059	3	0	1	0	3	0	4
39	M3	1	max	125.58	2	1.778	4	-.013	15	0	15	.001	1	0	4
40			min	-130.137	3	.418	15	-.347	1	0	1	0	15	0	15
41		2	max	125.511	2	1.601	4	-.013	15	0	15	.001	1	0	2
42			min	-130.189	3	.377	15	-.347	1	0	1	0	15	0	12
43		3	max	125.443	2	1.424	4	-.013	15	0	15	0	1	0	2
44			min	-130.24	3	.335	15	-.347	1	0	1	0	15	0	3
45		4	max	125.374	2	1.246	4	-.013	15	0	15	0	1	0	15
46			min	-130.292	3	.293	15	-.347	1	0	1	0	15	0	4
47		5	max	125.305	2	1.069	4	-.013	15	0	15	0	1	0	15
48			min	-130.343	3	.252	15	-.347	1	0	1	0	15	0	4
49		6	max	125.237	2	.892	4	-.013	15	0	15	0	1	0	15
50			min	-130.394	3	.21	15	-.347	1	0	1	0	15	0	4
51		7	max	125.168	2	.715	4	-.013	15	0	15	0	1	0	15
52			min	-130.446	3	.168	15	-.347	1	0	1	0	15	0	4
53		8	max	125.1	2	.538	4	-.013	15	0	15	0	1	0	15
54			min	-130.497	3	.127	15	-.347	1	0	1	0	15	-.001	4
55		9	max	125.031	2	.36	4	-.013	15	0	15	0	1	0	15
56			min	-130.549	3	.085	15	-.347	1	0	1	0	15	-.001	4
57		10	max	124.962	2	.183	4	-.013	15	0	15	0	1	0	15
58			min	-130.6	3	.043	15	-.347	1	0	1	0	15	-.001	4
59		11	max	124.894	2	.027	2	-.013	15	0	15	0	1	0	15
60			min	-130.652	3	-.022	3	-.347	1	0	1	0	15	-.001	4
61		12	max	124.825	2	-.04	15	-.013	15	0	15	0	1	0	15
62			min	-130.703	3	-.171	4	-.347	1	0	1	0	15	-.001	4
63		13	max	124.757	2	-.082	15	-.013	15	0	15	0	1	0	15
64			min	-130.755	3	-.348	4	-.347	1	0	1	0	15	-.001	4
65		14	max	124.688	2	-.123	15	-.013	15	0	15	0	1	0	15
66			min	-130.806	3	-.526	4	-.347	1	0	1	0	15	-.001	4
67		15	max	124.619	2	-.165	15	-.013	15	0	15	0	1	0	15
68			min	-130.858	3	-.703	4	-.347	1	0	1	0	10	0	4
69		16	max	124.551	2	-.207	15	-.013	15	0	15	0	15	0	15
70			min	-130.909	3	-.88	4	-.347	1	0	1	0	1	0	4
71		17	max	124.482	2	-.248	15	-.013	15	0	15	0	15	0	15
72			min	-130.96	3	-1.057	4	-.347	1	0	1	0	1	0	4
73		18	max	124.414	2	-.29	15	-.013	15	0	15	0	15	0	15
74			min	-131.012	3	-1.234	4	-.347	1	0	1	0	1	0	4
75		19	max	124.345	2	-.331	15	-.013	15	0	15	0	15	0	1
76			min	-131.063	3	-1.412	4	-.347	1	0	1	0	1	0	1
77	M4	1	max	388.13	1	0	1	-.035	15	0	1	0	3	0	1
78			min	-30.991	3	0	1	-.893	1	0	1	0	2	0	1
79		2	max	388.194	1	0	1	-.035	15	0	1	0	15	0	1
80			min	-30.942	3	0	1	-.893	1	0	1	0	1	0	1
81		3	max	388.259	1	0	1	-.035	15	0	1	0	15	0	1
82			min	-30.894	3	0	1	-.893	1	0	1	0	1	0	1
83		4	max	388.324	1	0	1	-.035	15	0	1	0	15	0	1
84			min	-30.845	3	0	1	-.893	1	0	1	0	1	0	1
85		5	max	388.388	1	0	1	-.035	15	0	1	0	15	0	1
86			min	-30.797	3	0	1	-.893	1	0	1	0	1	0	1
87		6	max	388.453	1	0	1	-.035	15	0	1	0	15	0	1
88			min	-30.748	3	0	1	-.893	1	0	1	0	1	0	1
89		7	max	388.518	1	0	1	-.035	15	0	1	0	15	0	1
90			min	-30.7	3	0	1	-.893	1	0	1	0	1	0	1
91		8	max	388.583	1	0	1	-.035	15	0	1	0	15	0	1
92			min	-30.651	3	0	1	-.893	1	0	1	0	1	0	1
93		9	max	388.647	1	0	1	-.035	15	0	1	0	15	0	1
94			min	-30.603	3	0	1	-.893	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	388.712	1	0	1	-.035	15	0	1	0	15	0	1
96		min	-30.554	3	0	1	-.893	1	0	1	0	1	0	1
97	11	max	388.777	1	0	1	-.035	15	0	1	0	15	0	1
98		min	-30.506	3	0	1	-.893	1	0	1	0	1	0	1
99	12	max	388.841	1	0	1	-.035	15	0	1	0	15	0	1
100		min	-30.457	3	0	1	-.893	1	0	1	0	1	0	1
101	13	max	388.906	1	0	1	-.035	15	0	1	0	15	0	1
102		min	-30.409	3	0	1	-.893	1	0	1	0	1	0	1
103	14	max	388.971	1	0	1	-.035	15	0	1	0	15	0	1
104		min	-30.36	3	0	1	-.893	1	0	1	-.001	1	0	1
105	15	max	389.036	1	0	1	-.035	15	0	1	0	15	0	1
106		min	-30.312	3	0	1	-.893	1	0	1	-.001	1	0	1
107	16	max	389.1	1	0	1	-.035	15	0	1	0	15	0	1
108		min	-30.263	3	0	1	-.893	1	0	1	-.001	1	0	1
109	17	max	389.165	1	0	1	-.035	15	0	1	0	15	0	1
110		min	-30.215	3	0	1	-.893	1	0	1	-.001	1	0	1
111	18	max	389.23	1	0	1	-.035	15	0	1	0	15	0	1
112		min	-30.166	3	0	1	-.893	1	0	1	-.001	1	0	1
113	19	max	389.294	1	0	1	-.035	15	0	1	0	15	0	1
114		min	-30.117	3	0	1	-.893	1	0	1	-.001	1	0	1
115	M6	1	max	880.356	1	.642	.12	1	0	3	0	3	0	1
116		min	-1170.202	3	.151	15	-.182	3	0	10	0	9	0	1
117	2	max	880.472	1	.596	4	.12	1	0	3	0	3	0	15
118		min	-1170.114	3	.141	15	-.182	3	0	10	0	9	0	4
119	3	max	880.588	1	.551	4	.12	1	0	3	0	3	0	15
120		min	-1170.027	3	.13	15	-.182	3	0	10	0	10	0	4
121	4	max	880.705	1	.505	4	.12	1	0	3	0	1	0	15
122		min	-1169.94	3	.119	15	-.182	3	0	10	0	10	0	4
123	5	max	880.821	1	.459	4	.12	1	0	3	0	1	0	15
124		min	-1169.852	3	.108	15	-.182	3	0	10	0	10	0	4
125	6	max	880.938	1	.414	4	.12	1	0	3	0	1	0	15
126		min	-1169.765	3	.098	15	-.182	3	0	10	0	3	0	4
127	7	max	881.054	1	.377	2	.12	1	0	3	0	1	0	15
128		min	-1169.678	3	.086	12	-.182	3	0	10	0	3	0	4
129	8	max	881.17	1	.342	2	.12	1	0	3	0	1	0	15
130		min	-1169.591	3	.068	12	-.182	3	0	10	0	3	0	4
131	9	max	881.287	1	.306	2	.12	1	0	3	0	1	0	15
132		min	-1169.503	3	.05	12	-.182	3	0	10	0	3	0	4
133	10	max	881.403	1	.271	2	.12	1	0	3	0	1	0	15
134		min	-1169.416	3	.032	12	-.182	3	0	10	0	3	0	4
135	11	max	881.52	1	.235	2	.12	1	0	3	0	1	0	15
136		min	-1169.329	3	.012	3	-.182	3	0	10	0	3	0	4
137	12	max	881.636	1	.2	2	.12	1	0	3	0	1	0	15
138		min	-1169.241	3	-.014	3	-.182	3	0	10	0	3	0	2
139	13	max	881.752	1	.164	2	.12	1	0	3	0	1	0	12
140		min	-1169.154	3	-.041	3	-.182	3	0	10	0	3	0	2
141	14	max	881.869	1	.128	2	.12	1	0	3	0	1	0	12
142		min	-1169.067	3	-.068	3	-.182	3	0	10	0	3	0	2
143	15	max	881.985	1	.093	2	.12	1	0	3	0	1	0	12
144		min	-1168.979	3	-.094	3	-.182	3	0	10	0	3	0	2
145	16	max	882.102	1	.057	2	.12	1	0	3	0	1	0	12
146		min	-1168.892	3	-.121	3	-.182	3	0	10	0	3	0	2
147	17	max	882.218	1	.022	2	.12	1	0	3	0	1	0	12
148		min	-1168.805	3	-.148	3	-.182	3	0	10	0	3	0	2
149	18	max	882.334	1	-.014	2	.12	1	0	3	0	1	0	12
150		min	-1168.718	3	-.175	3	-.182	3	0	10	0	3	0	2
151	19	max	882.451	1	-.042	15	.12	1	0	3	0	1	0	12



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1168.63	3	-.201	3	-.182	3	0	10	0	3	0	2
153	M7	1	max	474.621	2	1.78	4	.015	3	0	1	0	1	2
154		min	-390.419	3	.419	15	-.014	2	0	3	0	3	0	12
155		2	max	474.553	2	1.602	4	.015	3	0	1	0	1	2
156		min	-390.471	3	.377	15	-.014	2	0	3	0	3	0	3
157		3	max	474.484	2	1.425	4	.015	3	0	1	0	1	2
158		min	-390.522	3	.335	15	-.014	2	0	3	0	3	0	3
159		4	max	474.416	2	1.248	4	.015	3	0	1	0	1	2
160		min	-390.574	3	.294	15	-.014	2	0	3	0	3	0	3
161		5	max	474.347	2	1.071	4	.015	3	0	1	0	1	15
162		min	-390.625	3	.252	15	-.014	2	0	3	0	3	0	3
163		6	max	474.278	2	.894	4	.015	3	0	1	0	1	15
164		min	-390.677	3	.21	15	-.014	2	0	3	0	3	0	4
165		7	max	474.21	2	.716	4	.015	3	0	1	0	1	15
166		min	-390.728	3	.169	15	-.014	2	0	3	0	3	0	4
167		8	max	474.141	2	.539	4	.015	3	0	1	0	1	15
168		min	-390.779	3	.127	15	-.014	2	0	3	0	3	-.001	4
169		9	max	474.073	2	.362	4	.015	3	0	1	0	1	15
170		min	-390.831	3	.085	15	-.014	2	0	3	0	3	-.001	4
171		10	max	474.004	2	.223	2	.015	3	0	1	0	1	15
172		min	-390.882	3	.021	12	-.014	2	0	3	0	3	-.001	4
173		11	max	473.935	2	.085	2	.015	3	0	1	0	1	15
174		min	-390.934	3	-.081	3	-.014	2	0	3	0	3	-.001	4
175		12	max	473.867	2	-.04	15	.015	3	0	1	0	1	15
176		min	-390.985	3	-.184	3	-.014	2	0	3	0	3	-.001	4
177		13	max	473.798	2	-.081	15	.015	3	0	1	0	1	15
178		min	-391.037	3	-.347	4	-.014	2	0	3	0	3	-.001	4
179		14	max	473.73	2	-.123	15	.015	3	0	1	0	1	15
180		min	-391.088	3	-.524	4	-.014	2	0	3	0	3	-.001	4
181		15	max	473.661	2	-.165	15	.015	3	0	1	0	1	15
182		min	-391.14	3	-.701	4	-.014	2	0	3	0	3	0	4
183		16	max	473.592	2	-.206	15	.015	3	0	1	0	1	15
184		min	-391.191	3	-.878	4	-.014	2	0	3	0	3	0	4
185		17	max	473.524	2	-.248	15	.015	3	0	1	0	1	15
186		min	-391.243	3	-1.056	4	-.014	2	0	3	0	3	0	4
187		18	max	473.455	2	-.29	15	.015	3	0	1	0	1	15
188		min	-391.294	3	-1.233	4	-.014	2	0	3	0	3	0	4
189		19	max	473.387	2	-.331	15	.015	3	0	1	0	1	1
190		min	-391.345	3	-1.41	4	-.014	2	0	3	0	3	0	1
191	M8	1	max	1080.171	1	0	1	.595	1	0	1	0	10	1
192		min	-131.375	3	0	1	-.434	3	0	1	0	1	0	1
193		2	max	1080.235	1	0	1	.595	1	0	1	0	1	1
194		min	-131.326	3	0	1	-.434	3	0	1	0	3	0	1
195		3	max	1080.3	1	0	1	.595	1	0	1	0	1	1
196		min	-131.278	3	0	1	-.434	3	0	1	0	3	0	1
197		4	max	1080.365	1	0	1	.595	1	0	1	0	1	1
198		min	-131.229	3	0	1	-.434	3	0	1	0	3	0	1
199		5	max	1080.43	1	0	1	.595	1	0	1	0	1	1
200		min	-131.181	3	0	1	-.434	3	0	1	0	3	0	1
201		6	max	1080.494	1	0	1	.595	1	0	1	0	1	1
202		min	-131.132	3	0	1	-.434	3	0	1	0	3	0	1
203		7	max	1080.559	1	0	1	.595	1	0	1	0	1	1
204		min	-131.084	3	0	1	-.434	3	0	1	0	3	0	1
205		8	max	1080.624	1	0	1	.595	1	0	1	0	1	1
206		min	-131.035	3	0	1	-.434	3	0	1	0	3	0	1
207		9	max	1080.688	1	0	1	.595	1	0	1	0	1	1
208		min	-130.987	3	0	1	-.434	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1080.753	1	0	1	.595	1	0	1	0	1	0	1
210			min	-130.938	3	0	1	-.434	3	0	1	0	3	0	1
211		11	max	1080.818	1	0	1	.595	1	0	1	0	1	0	1
212			min	-130.89	3	0	1	-.434	3	0	1	0	3	0	1
213		12	max	1080.883	1	0	1	.595	1	0	1	0	1	0	1
214			min	-130.841	3	0	1	-.434	3	0	1	0	3	0	1
215		13	max	1080.947	1	0	1	.595	1	0	1	0	1	0	1
216			min	-130.793	3	0	1	-.434	3	0	1	0	3	0	1
217		14	max	1081.012	1	0	1	.595	1	0	1	0	1	0	1
218			min	-130.744	3	0	1	-.434	3	0	1	0	3	0	1
219		15	max	1081.077	1	0	1	.595	1	0	1	0	1	0	1
220			min	-130.696	3	0	1	-.434	3	0	1	0	3	0	1
221		16	max	1081.141	1	0	1	.595	1	0	1	0	1	0	1
222			min	-130.647	3	0	1	-.434	3	0	1	0	3	0	1
223		17	max	1081.206	1	0	1	.595	1	0	1	0	1	0	1
224			min	-130.599	3	0	1	-.434	3	0	1	0	3	0	1
225		18	max	1081.271	1	0	1	.595	1	0	1	0	1	0	1
226			min	-130.55	3	0	1	-.434	3	0	1	0	3	0	1
227		19	max	1081.335	1	0	1	.595	1	0	1	0	1	0	1
228			min	-130.501	3	0	1	-.434	3	0	1	0	3	0	1
229	M10	1	max	278.299	1	.637	4	-.004	15	0	1	0	1	0	1
230			min	-338.975	3	.151	15	-.126	1	0	3	0	3	0	1
231		2	max	278.416	1	.591	4	-.004	15	0	1	0	1	0	15
232			min	-338.888	3	.14	15	-.126	1	0	3	0	3	0	4
233		3	max	278.532	1	.546	4	-.004	15	0	1	0	1	0	15
234			min	-338.801	3	.129	15	-.126	1	0	3	0	3	0	4
235		4	max	278.649	1	.5	4	-.004	15	0	1	0	1	0	15
236			min	-338.714	3	.118	15	-.126	1	0	3	0	3	0	4
237		5	max	278.765	1	.454	4	-.004	15	0	1	0	1	0	15
238			min	-338.626	3	.108	15	-.126	1	0	3	0	3	0	4
239		6	max	278.881	1	.409	4	-.004	15	0	1	0	1	0	15
240			min	-338.539	3	.097	15	-.126	1	0	3	0	3	0	4
241		7	max	278.998	1	.363	4	-.004	15	0	1	0	1	0	15
242			min	-338.452	3	.086	15	-.126	1	0	3	0	3	0	4
243		8	max	279.114	1	.317	4	-.004	15	0	1	0	1	0	15
244			min	-338.364	3	.076	15	-.126	1	0	3	0	3	0	4
245		9	max	279.231	1	.272	4	-.004	15	0	1	0	9	0	15
246			min	-338.277	3	.065	15	-.126	1	0	3	0	3	0	4
247		10	max	279.347	1	.226	4	-.004	15	0	1	0	15	0	15
248			min	-338.19	3	.054	15	-.126	1	0	3	0	3	0	4
249		11	max	279.463	1	.18	4	-.004	15	0	1	0	15	0	15
250			min	-338.102	3	.043	15	-.126	1	0	3	0	3	0	4
251		12	max	279.58	1	.138	2	-.004	15	0	1	0	15	0	15
252			min	-338.015	3	.033	15	-.126	1	0	3	0	3	0	4
253		13	max	279.696	1	.102	2	-.004	15	0	1	0	15	0	15
254			min	-337.928	3	.022	15	-.126	1	0	3	0	3	0	4
255		14	max	279.813	1	.067	2	-.004	15	0	1	0	15	0	15
256			min	-337.841	3	.011	15	-.126	1	0	3	0	3	0	4
257		15	max	279.929	1	.031	2	-.004	15	0	1	0	15	0	15
258			min	-337.753	3	-.018	9	-.126	1	0	3	0	3	0	4
259		16	max	280.045	1	-.004	2	-.004	15	0	1	0	15	0	15
260			min	-337.666	3	-.048	14	-.126	1	0	3	0	3	0	4
261		17	max	280.162	1	-.021	15	-.004	15	0	1	0	15	0	15
262			min	-337.579	3	-.094	4	-.126	1	0	3	0	1	0	4
263		18	max	280.278	1	-.032	15	-.004	15	0	1	0	15	0	15
264			min	-337.491	3	-.139	4	-.126	1	0	3	0	1	0	4
265		19	max	280.395	1	-.043	15	-.004	15	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-337.404	3	-.185	4	-.126	1	0	3	0	1	0	4
267		1	max	125.206	2	1.782	4	.386	1	0	1	0	3	0	4
268			min	-130.762	3	.419	15	-.015	3	0	15	-.001	1	0	15
269		2	max	125.137	2	1.604	4	.386	1	0	1	0	3	0	4
270			min	-130.813	3	.377	15	-.015	3	0	15	-.001	1	0	12
271		3	max	125.068	2	1.427	4	.386	1	0	1	0	3	0	2
272			min	-130.865	3	.336	15	-.015	3	0	15	0	1	0	3
273		4	max	125	2	1.25	4	.386	1	0	1	0	3	0	15
274			min	-130.916	3	.294	15	-.015	3	0	15	0	1	0	3
275		5	max	124.931	2	1.073	4	.386	1	0	1	0	3	0	15
276			min	-130.968	3	.252	15	-.015	3	0	15	0	1	0	4
277		6	max	124.863	2	.896	4	.386	1	0	1	0	3	0	15
278			min	-131.019	3	.211	15	-.015	3	0	15	0	1	0	4
279		7	max	124.794	2	.718	4	.386	1	0	1	0	3	0	15
280			min	-131.071	3	.169	15	-.015	3	0	15	0	1	0	4
281		8	max	124.725	2	.541	4	.386	1	0	1	0	3	0	15
282			min	-131.122	3	.127	15	-.015	3	0	15	0	1	-.001	4
283	9	max	124.657	2	.364	4	.386	1	0	1	0	3	0	15	
284		min	-131.173	3	.086	15	-.015	3	0	15	0	1	-.001	4	
285	10	max	124.588	2	.187	4	.386	1	0	1	0	3	0	15	
286		min	-131.225	3	.044	15	-.015	3	0	15	0	1	-.001	4	
287	11	max	124.52	2	.027	2	.386	1	0	1	0	3	0	15	
288		min	-131.276	3	-.038	3	-.015	3	0	15	0	1	-.001	4	
289	12	max	124.451	2	-.039	15	.386	1	0	1	0	3	0	15	
290		min	-131.328	3	-.168	4	-.015	3	0	15	0	1	-.001	4	
291	13	max	124.382	2	-.081	15	.386	1	0	1	0	3	0	15	
292		min	-131.379	3	-.345	4	-.015	3	0	15	0	1	-.001	4	
293	14	max	124.314	2	-.123	15	.386	1	0	1	0	3	0	15	
294		min	-131.431	3	-.522	4	-.015	3	0	15	0	1	-.001	4	
295	15	max	124.245	2	-.164	15	.386	1	0	1	0	3	0	15	
296		min	-131.482	3	-.699	4	-.015	3	0	15	0	10	0	4	
297	16	max	124.177	2	-.206	15	.386	1	0	1	0	3	0	15	
298		min	-131.534	3	-.876	4	-.015	3	0	15	0	15	0	4	
299	17	max	124.108	2	-.248	15	.386	1	0	1	0	3	0	15	
300		min	-131.585	3	-1.054	4	-.015	3	0	15	0	15	0	4	
301	18	max	124.039	2	-.289	15	.386	1	0	1	0	1	0	15	
302		min	-131.637	3	-1.231	4	-.015	3	0	15	0	15	0	4	
303	19	max	123.971	2	-.331	15	.386	1	0	1	0	1	0	1	
304		min	-131.688	3	-1.408	4	-.015	3	0	15	0	15	0	1	
305	M12	1	max	387.934	1	0	1	2.109	1	0	1	0	2	0	1
306		min	-30.513	3	0	1	.079	15	0	1	0	3	0	1	
307	2	max	387.999	1	0	1	2.109	1	0	1	0	1	0	1	
308		min	-30.464	3	0	1	.079	15	0	1	0	15	0	1	
309	3	max	388.064	1	0	1	2.109	1	0	1	0	1	0	1	
310		min	-30.416	3	0	1	.079	15	0	1	0	15	0	1	
311	4	max	388.128	1	0	1	2.109	1	0	1	0	1	0	1	
312		min	-30.367	3	0	1	.079	15	0	1	0	15	0	1	
313	5	max	388.193	1	0	1	2.109	1	0	1	0	1	0	1	
314		min	-30.319	3	0	1	.079	15	0	1	0	15	0	1	
315	6	max	388.258	1	0	1	2.109	1	0	1	0	1	0	1	
316		min	-30.27	3	0	1	.079	15	0	1	0	15	0	1	
317	7	max	388.323	1	0	1	2.109	1	0	1	.001	1	0	1	
318		min	-30.222	3	0	1	.079	15	0	1	0	15	0	1	
319	8	max	388.387	1	0	1	2.109	1	0	1	.001	1	0	1	
320		min	-30.173	3	0	1	.079	15	0	1	0	15	0	1	
321	9	max	388.452	1	0	1	2.109	1	0	1	.002	1	0	1	
322		min	-30.125	3	0	1	.079	15	0	1	0	15	0	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	388.517	1	0	1	2.109	1	0	1	.002	1	0	1
324		min	-30.076	3	0	1	.079	15	0	1	0	15	0	1
325	11	max	388.581	1	0	1	2.109	1	0	1	.002	1	0	1
326		min	-30.028	3	0	1	.079	15	0	1	0	15	0	1
327	12	max	388.646	1	0	1	2.109	1	0	1	.002	1	0	1
328		min	-29.979	3	0	1	.079	15	0	1	0	15	0	1
329	13	max	388.711	1	0	1	2.109	1	0	1	.002	1	0	1
330		min	-29.931	3	0	1	.079	15	0	1	0	15	0	1
331	14	max	388.775	1	0	1	2.109	1	0	1	.002	1	0	1
332		min	-29.882	3	0	1	.079	15	0	1	0	15	0	1
333	15	max	388.84	1	0	1	2.109	1	0	1	.003	1	0	1
334		min	-29.834	3	0	1	.079	15	0	1	0	15	0	1
335	16	max	388.905	1	0	1	2.109	1	0	1	.003	1	0	1
336		min	-29.785	3	0	1	.079	15	0	1	0	15	0	1
337	17	max	388.97	1	0	1	2.109	1	0	1	.003	1	0	1
338		min	-29.737	3	0	1	.079	15	0	1	0	15	0	1
339	18	max	389.034	1	0	1	2.109	1	0	1	.003	1	0	1
340		min	-29.688	3	0	1	.079	15	0	1	0	15	0	1
341	19	max	389.099	1	0	1	2.109	1	0	1	.003	1	0	1
342		min	-29.639	3	0	1	.079	15	0	1	0	15	0	1
343	M1	1	max	106.525	1	346.524	3	-1.547	15	0	.081	1	.013	1
344		min	3.93	15	-277.133	1	-41.025	1	0	3	.003	15	-.014	3
345	2	max	106.643	1	346.334	3	-1.547	15	0	1	.072	1	.073	1
346		min	3.965	15	-277.386	1	-41.025	1	0	3	.003	15	-.089	3
347	3	max	68.83	1	5.633	9	-1.534	15	0	12	.062	1	.133	1
348		min	-5.565	10	-20.229	2	-40.856	1	0	1	.002	15	-.163	3
349	4	max	68.948	1	5.422	9	-1.534	15	0	12	.053	1	.133	1
350		min	-5.466	10	-20.482	2	-40.856	1	0	1	.002	15	-.158	3
351	5	max	69.066	1	5.211	9	-1.534	15	0	12	.044	1	.134	1
352		min	-5.368	10	-20.735	2	-40.856	1	0	1	.002	15	-.154	3
353	6	max	69.184	1	5	9	-1.534	15	0	12	.036	1	.136	2
354		min	-5.27	10	-20.988	2	-40.856	1	0	1	.001	15	-.15	3
355	7	max	69.302	1	4.789	9	-1.534	15	0	12	.027	1	.14	2
356		min	-5.171	10	-21.241	2	-40.856	1	0	1	.001	15	-.146	3
357	8	max	69.42	1	4.579	9	-1.534	15	0	12	.018	1	.145	2
358		min	-5.073	10	-21.494	2	-40.856	1	0	1	0	15	-.141	3
359	9	max	69.538	1	4.368	9	-1.534	15	0	12	.009	1	.15	2
360		min	-4.975	10	-21.747	2	-40.856	1	0	1	0	15	-.137	3
361	10	max	69.656	1	4.157	9	-1.534	15	0	12	.001	3	.154	2
362		min	-4.876	10	-22	2	-40.856	1	0	1	0	15	-.132	3
363	11	max	69.774	1	3.946	9	-1.534	15	0	12	0	3	.159	2
364		min	-4.778	10	-22.254	2	-40.856	1	0	1	-.009	1	-.128	3
365	12	max	69.892	1	3.735	9	-1.534	15	0	12	0	12	.164	2
366		min	-4.68	10	-22.507	2	-40.856	1	0	1	-.018	1	-.123	3
367	13	max	70.01	1	3.524	9	-1.534	15	0	12	0	15	.169	2
368		min	-4.581	10	-22.76	2	-40.856	1	0	1	-.026	1	-.119	3
369	14	max	70.128	1	3.313	9	-1.534	15	0	12	-.001	15	.174	2
370		min	-4.483	10	-23.013	2	-40.856	1	0	1	-.035	1	-.114	3
371	15	max	70.246	1	3.102	9	-1.534	15	0	12	-.002	15	.179	2
372		min	-4.385	10	-23.266	2	-40.856	1	0	1	-.044	1	-.109	3
373	16	max	88.638	2	73.374	2	-1.547	15	0	1	-.002	15	.183	2
374		min	-19.831	3	-124.796	3	-41.159	1	0	12	-.053	1	-.104	3
375	17	max	88.756	2	73.121	2	-1.547	15	0	1	-.002	15	.167	2
376		min	-19.743	3	-124.986	3	-41.159	1	0	12	-.062	1	-.077	3
377	18	max	-3.959	15	350.422	2	-1.583	15	0	3	-.003	15	.092	2
378		min	-106.624	1	-153.304	3	-42.118	1	0	2	-.071	1	-.043	3
379	19	max	-3.924	15	350.168	2	-1.583	15	0	3	-.003	15	.016	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-106.506	1	-153.494	3	-42.118	1	0	2	-.081	1	-.01	3
381	M5	1	max	245.261	1	1126.707	3	0	10	0	.007	3	.028	3
382		min	5.896	12	-899.198	1	-49.813	3	0	3	0	10	-.027	1
383		2	max	245.379	1	1126.517	3	0	10	0	0	1	.169	1
384		min	5.955	12	-899.451	1	-49.813	3	0	3	-.004	3	-.216	3
385		3	max	175.699	3	6.459	9	5.482	3	0	0	1	.36	1
386		min	-24.092	10	-68.609	2	-.643	1	0	1	-.014	3	-.456	3
387		4	max	175.787	3	6.248	9	5.482	3	0	0	1	.366	1
388		min	-23.994	10	-68.862	2	-.643	1	0	1	-.013	3	-.442	3
389		5	max	175.876	3	6.037	9	5.482	3	0	0	1	.372	1
390		min	-23.895	10	-69.115	2	-.643	1	0	1	-.012	3	-.428	3
391		6	max	175.964	3	5.826	9	5.482	3	0	0	1	.378	1
392		min	-23.797	10	-69.368	2	-.643	1	0	1	-.011	3	-.415	3
393		7	max	176.053	3	5.616	9	5.482	3	0	0	1	.386	2
394		min	-23.699	10	-69.621	2	-.643	1	0	1	-.01	3	-.401	3
395		8	max	176.141	3	5.405	9	5.482	3	0	0	1	.401	2
396		min	-23.6	10	-69.874	2	-.643	1	0	1	-.008	3	-.387	3
397		9	max	176.23	3	5.194	9	5.482	3	0	0	1	.417	2
398		min	-23.502	10	-70.127	2	-.643	1	0	1	-.007	3	-.373	3
399		10	max	176.318	3	4.983	9	5.482	3	0	0	10	.432	2
400		min	-23.404	10	-70.38	2	-.643	1	0	1	-.006	3	-.359	3
401		11	max	176.407	3	4.772	9	5.482	3	0	0	10	.447	2
402		min	-23.305	10	-70.633	2	-.643	1	0	1	-.005	3	-.346	3
403		12	max	176.495	3	4.561	9	5.482	3	0	0	10	.462	2
404		min	-23.207	10	-70.887	2	-.643	1	0	1	-.004	3	-.332	3
405		13	max	176.584	3	4.35	9	5.482	3	0	0	10	.478	2
406		min	-23.109	10	-71.14	2	-.643	1	0	1	-.002	3	-.318	3
407		14	max	176.672	3	4.139	9	5.482	3	0	0	10	.493	2
408		min	-23.01	10	-71.393	2	-.643	1	0	1	-.001	3	-.304	3
409		15	max	176.761	3	3.928	9	5.482	3	0	0	10	.509	2
410		min	-22.912	10	-71.646	2	-.643	1	0	1	0	1	-.29	3
411		16	max	292.247	2	300.872	2	5.455	3	0	0	3	.521	2
412		min	-66.631	3	-371.163	3	-.649	1	0	10	0	1	-.273	3
413		17	max	292.365	2	300.619	2	5.455	3	0	.002	3	.456	2
414		min	-66.542	3	-371.353	3	-.649	1	0	10	0	1	-.192	3
415		18	max	-7.519	12	1133.641	2	5.024	3	0	.003	3	.213	2
416		min	-245.411	1	-491.084	3	-.148	1	0	1	0	1	-.086	3
417		19	max	-7.46	12	1133.388	2	5.024	3	0	.004	3	.02	3
418		min	-245.292	1	-491.274	3	-.148	1	0	1	0	1	-.033	2
419	M9	1	max	106.139	1	346.489	3	53.9	3	0	-.003	15	.013	1
420		min	3.912	15	-277.131	1	1.72	15	0	1	-.079	1	-.014	3
421		2	max	106.257	1	346.299	3	53.9	3	0	-.002	12	.073	1
422		min	3.948	15	-277.384	1	1.72	15	0	1	-.07	1	-.089	3
423		3	max	68.941	1	5.606	9	39.824	1	0	.009	3	.132	1
424		min	-5.137	10	-20.244	2	-.948	3	0	15	-.061	1	-.163	3
425		4	max	69.059	1	5.395	9	39.824	1	0	.009	3	.133	1
426		min	-5.039	10	-20.497	2	-.948	3	0	15	-.052	1	-.158	3
427		5	max	69.177	1	5.184	9	39.824	1	0	.009	3	.134	1
428		min	-4.94	10	-20.75	2	-.948	3	0	15	-.043	1	-.154	3
429		6	max	69.295	1	4.973	9	39.824	1	0	.008	3	.136	2
430		min	-4.842	10	-21.003	2	-.948	3	0	15	-.035	1	-.15	3
431		7	max	69.413	1	4.762	9	39.824	1	0	.008	3	.14	2
432		min	-4.744	10	-21.256	2	-.948	3	0	15	-.026	1	-.146	3
433		8	max	69.531	1	4.551	9	39.824	1	0	.008	3	.145	2
434		min	-4.645	10	-21.509	2	-.948	3	0	15	-.017	1	-.141	3
435		9	max	69.649	1	4.34	9	39.824	1	0	.008	3	.149	2
436		min	-4.547	10	-21.763	2	-.948	3	0	15	-.009	1	-.137	3

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	69.767	1	4.129	9	39.824	1	0	1	.008	3	.154	2
438			min	-4.449	10	-22.016	2	-.948	3	0	15	0	1	-.132	3
439		11	max	69.885	1	3.919	9	39.824	1	0	1	.009	1	.159	2
440			min	-4.35	10	-22.269	2	-.948	3	0	15	0	15	-.128	3
441		12	max	70.003	1	3.708	9	39.824	1	0	1	.017	1	.164	2
442			min	-4.252	10	-22.522	2	-.948	3	0	15	0	15	-.123	3
443		13	max	70.121	1	3.497	9	39.824	1	0	1	.026	1	.169	2
444			min	-4.154	10	-22.775	2	-.948	3	0	15	0	15	-.119	3
445		14	max	70.239	1	3.286	9	39.824	1	0	1	.034	1	.174	2
446			min	-4.055	10	-23.028	2	-.948	3	0	15	.001	15	-.114	3
447		15	max	70.357	1	3.075	9	39.824	1	0	1	.043	1	.179	2
448			min	-3.957	10	-23.281	2	-.948	3	0	15	.002	15	-.109	3
449		16	max	88.843	2	73.067	2	40.169	1	0	15	.052	1	.183	2
450			min	-20.2	3	-125.226	3	-.955	3	0	1	.002	15	-.104	3
451		17	max	88.961	2	72.814	2	40.169	1	0	15	.061	1	.167	2
452			min	-20.111	3	-125.416	3	-.955	3	0	1	.002	15	-.077	3
453		18	max	-3.945	15	350.422	2	42.277	1	0	2	.07	1	.092	2
454			min	-106.245	1	-153.3	3	-.566	3	0	3	.003	15	-.043	3
455		19	max	-3.91	15	350.169	2	42.277	1	0	2	.079	1	.016	2
456			min	-106.127	1	-153.489	3	-.566	3	0	3	.003	15	-.01	3
457	M13	1	max	53.897	3	276.755	1	-3.912	15	.013	1	.079	1	0	1
458			min	1.72	15	-346.493	3	-106.132	1	-.014	3	.003	15	0	3
459		2	max	53.897	3	196.196	1	-2.982	15	.013	1	.02	1	.189	3
460			min	1.72	15	-245.406	3	-80.692	1	-.014	3	0	10	-.151	1
461		3	max	53.897	3	115.638	1	-2.051	15	.013	1	.006	3	.314	3
462			min	1.72	15	-144.319	3	-55.251	1	-.014	3	-.024	1	-.251	1
463		4	max	53.897	3	35.079	1	-1.121	15	.013	1	.003	3	.373	3
464			min	1.72	15	-43.233	3	-29.811	1	-.014	3	-.051	1	-.299	1
465		5	max	53.897	3	57.854	3	.407	10	.013	1	0	3	.369	3
466			min	1.72	15	-45.48	1	-4.371	1	-.014	3	-.062	1	-.296	1
467		6	max	53.897	3	158.94	3	21.07	1	.013	1	0	3	.3	3
468			min	1.72	15	-126.039	1	-1.055	3	-.014	3	-.056	1	-.241	1
469		7	max	53.897	3	260.027	3	46.51	1	.013	1	0	12	.166	3
470			min	1.72	15	-206.598	1	.307	3	-.014	3	-.035	1	-.134	1
471		8	max	53.897	3	361.114	3	71.95	1	.013	1	.003	2	.023	1
472			min	1.72	15	-287.157	1	1.255	12	-.014	3	0	3	-.033	3
473		9	max	53.897	3	462.2	3	97.391	1	.013	1	.057	1	.232	1
474			min	1.72	15	-367.716	1	2.163	12	-.014	3	.001	12	-.296	3
475		10	max	53.897	3	563.287	3	122.831	1	.013	1	.127	1	.493	1
476			min	1.72	15	-448.274	1	3.071	12	-.014	3	.003	12	-.623	3
477		11	max	41.128	1	367.716	1	-1.913	12	.014	3	.056	1	.232	1
478			min	1.548	15	-462.2	3	-97.005	1	-.013	1	-.005	3	-.296	3
479		12	max	41.128	1	287.157	1	-1.005	12	.014	3	.003	2	.023	1
480			min	1.548	15	-361.114	3	-71.564	1	-.013	1	-.006	3	-.033	3
481		13	max	41.128	1	206.598	1	.105	3	.014	3	-.001	15	.166	3
482			min	1.548	15	-260.027	3	-46.124	1	-.013	1	-.035	1	-.134	1
483		14	max	41.128	1	126.039	1	1.467	3	.014	3	-.002	15	.3	3
484			min	1.548	15	-158.94	3	-20.684	1	-.013	1	-.057	1	-.241	1
485		15	max	41.128	1	45.48	1	4.757	1	.014	3	-.002	15	.369	3
486			min	1.548	15	-57.854	3	-.406	10	-.013	1	-.062	1	-.296	1
487		16	max	41.128	1	43.233	3	30.197	1	.014	3	-.002	12	.373	3
488			min	1.548	15	-35.079	1	1.138	15	-.013	1	-.05	1	-.299	1
489		17	max	41.128	1	144.319	3	55.637	1	.014	3	0	3	.314	3
490			min	1.548	15	-115.638	1	2.069	15	-.013	1	-.023	1	-.251	1
491		18	max	41.128	1	245.406	3	81.078	1	.014	3	.021	1	.189	3
492			min	1.548	15	-196.196	1	2.999	15	-.013	1	0	10	-.151	1
493		19	max	41.128	1	346.493	3	106.518	1	.014	3	.081	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	1.548	15	-276.755	1	3.93	15	-.013	1	.003	15	0	3
495	M16	1	max	.569	3	350.347	2	-3.91	15	.01	3	.079	1	0	2
496			min	-42.169	1	-153.512	3	-106.135	1	-.016	2	.003	15	0	3
497		2	max	.569	3	248.38	2	-2.979	15	.01	3	.02	1	.084	3
498			min	-42.169	1	-109.157	3	-80.694	1	-.016	2	0	10	-.191	2
499		3	max	.569	3	146.414	2	-2.049	15	.01	3	0	12	.139	3
500			min	-42.169	1	-64.802	3	-55.254	1	-.016	2	-.024	1	-.317	2
501		4	max	.569	3	44.447	2	-1.118	15	.01	3	-.002	15	.167	3
502			min	-42.169	1	-20.447	3	-29.814	1	-.016	2	-.051	1	-.378	2
503		5	max	.569	3	23.908	3	.399	10	.01	3	-.002	15	.166	3
504			min	-42.169	1	-57.52	2	-4.373	1	-.016	2	-.062	1	-.374	2
505		6	max	.569	3	68.264	3	21.067	1	.01	3	-.002	15	.136	3
506			min	-42.169	1	-159.486	2	-.241	3	-.016	2	-.056	1	-.305	2
507		7	max	.569	3	112.619	3	46.507	1	.01	3	-.001	15	.078	3
508			min	-42.169	1	-261.453	2	.836	12	-.016	2	-.035	1	-.17	2
509		8	max	.569	3	156.974	3	71.948	1	.01	3	.003	2	.029	2
510			min	-42.169	1	-363.42	2	1.744	12	-.016	2	-.004	3	-.008	3
511		9	max	.569	3	201.329	3	97.388	1	.01	3	.057	1	.294	2
512			min	-42.169	1	-465.386	2	2.652	12	-.016	2	-.002	3	-.122	3
513		10	max	-1.582	15	-11.111	15	122.829	1	0	15	.127	1	.624	2
514			min	-42.169	1	-567.353	2	-5.778	3	-.016	2	.003	12	-.265	3
515		11	max	-1.582	15	465.386	2	-2.992	12	.016	2	.056	1	.294	2
516			min	-42.016	1	-201.329	3	-97.009	1	-.01	3	.001	12	-.122	3
517		12	max	-1.582	15	363.42	2	-2.084	12	.016	2	.003	2	.029	2
518			min	-42.016	1	-156.974	3	-71.568	1	-.01	3	0	3	-.008	3
519		13	max	-1.582	15	261.453	2	-1.176	12	.016	2	-.001	15	.078	3
520			min	-42.016	1	-112.619	3	-46.128	1	-.01	3	-.035	1	-.17	2
521		14	max	-1.582	15	159.486	2	-.268	12	.016	2	-.002	12	.136	3
522			min	-42.016	1	-68.264	3	-20.688	1	-.01	3	-.056	1	-.305	2
523		15	max	-1.582	15	57.52	2	4.753	1	.016	2	-.002	12	.166	3
524			min	-42.016	1	-23.908	3	-.399	10	-.01	3	-.062	1	-.374	2
525		16	max	-1.582	15	20.447	3	30.193	1	.016	2	-.001	12	.167	3
526			min	-42.016	1	-44.447	2	1.132	15	-.01	3	-.05	1	-.378	2
527		17	max	-1.582	15	64.802	3	55.634	1	.016	2	0	3	.139	3
528			min	-42.016	1	-146.414	2	2.063	15	-.01	3	-.023	1	-.317	2
529		18	max	-1.582	15	109.157	3	81.074	1	.016	2	.021	1	.084	3
530			min	-42.016	1	-248.381	2	2.993	15	-.01	3	0	10	-.191	2
531		19	max	-1.582	15	153.512	3	106.514	1	.016	2	.081	1	0	2
532			min	-42.016	1	-350.347	2	3.924	15	-.01	3	.003	15	0	3
533	M15	1	max	.199	13	2.016	4	.062	3	0	9	0	9	0	1
534			min	-62.246	3	0	1	-.022	9	0	3	0	3	0	1
535		2	max	.11	13	1.792	4	.062	3	0	9	0	9	0	1
536			min	-62.311	3	0	1	-.022	9	0	3	0	3	0	4
537		3	max	.02	13	1.568	4	.062	3	0	9	0	9	0	1
538			min	-62.376	3	0	1	-.022	9	0	3	0	3	-.001	4
539		4	max	0	1	1.344	4	.062	3	0	9	0	9	0	1
540			min	-62.441	3	0	1	-.022	9	0	3	0	3	-.002	4
541		5	max	0	1	1.12	4	.062	3	0	9	0	9	0	1
542			min	-62.507	3	0	1	-.022	9	0	3	0	3	-.002	4
543		6	max	0	1	.896	4	.062	3	0	9	0	9	0	1
544			min	-62.572	3	0	1	-.022	9	0	3	0	3	-.003	4
545		7	max	0	1	.672	4	.062	3	0	9	0	3	0	1
546			min	-62.637	3	0	1	-.022	9	0	3	0	9	-.003	4
547		8	max	0	1	.448	4	.062	3	0	9	0	3	0	1
548			min	-62.702	3	0	1	-.022	9	0	3	0	9	-.003	4
549		9	max	0	1	.224	4	.062	3	0	9	0	3	0	1
550			min	-62.767	3	0	1	-.022	9	0	3	0	9	-.003	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.062	3	0	9	0	3	0	1
552		min	-62.833	3	0	1	-.022	9	0	3	0	9	-.003	4
553	11	max	0	1	0	1	.062	3	0	9	0	3	0	1
554		min	-62.898	3	-.224	4	-.022	9	0	3	0	9	-.003	4
555	12	max	0	1	0	1	.062	3	0	9	0	3	0	1
556		min	-62.963	3	-.448	4	-.022	9	0	3	0	9	-.003	4
557	13	max	0	1	0	1	.062	3	0	9	0	3	0	1
558		min	-63.028	3	-.672	4	-.022	9	0	3	0	9	-.003	4
559	14	max	0	1	0	1	.062	3	0	9	0	3	0	1
560		min	-63.093	3	-.896	4	-.022	9	0	3	0	9	-.003	4
561	15	max	0	1	0	1	.062	3	0	9	0	3	0	1
562		min	-63.158	3	-1.12	4	-.022	9	0	3	0	9	-.002	4
563	16	max	0	1	0	1	.062	3	0	9	0	3	0	1
564		min	-63.224	3	-1.344	4	-.022	9	0	3	0	9	-.002	4
565	17	max	0	1	0	1	.062	3	0	9	0	3	0	1
566		min	-63.289	3	-1.568	4	-.022	9	0	3	0	9	-.001	4
567	18	max	0	1	0	1	.062	3	0	9	0	3	0	1
568		min	-63.354	3	-1.792	4	-.022	9	0	3	0	9	0	4
569	19	max	0	1	0	1	.062	3	0	9	0	3	0	1
570		min	-63.419	3	-2.016	4	-.022	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.016	.031	1	0	3	0	3	0	1
572		min	-62.55	3	0	10	-.028	3	0	2	0	1	0	1
573	2	max	0	10	1.792	4	.031	1	0	3	0	3	0	10
574		min	-62.485	3	0	10	-.028	3	0	2	0	1	0	4
575	3	max	0	10	1.568	4	.031	1	0	3	0	3	0	10
576		min	-62.42	3	0	10	-.028	3	0	2	0	1	-.001	4
577	4	max	0	10	1.344	4	.031	1	0	3	0	3	0	10
578		min	-62.355	3	0	10	-.028	3	0	2	0	1	-.002	4
579	5	max	0	10	1.12	4	.031	1	0	3	0	3	0	10
580		min	-62.29	3	0	10	-.028	3	0	2	0	1	-.002	4
581	6	max	0	10	.896	4	.031	1	0	3	0	3	0	10
582		min	-62.224	3	0	10	-.028	3	0	2	0	1	-.003	4
583	7	max	0	10	.672	4	.031	1	0	3	0	3	0	10
584		min	-62.159	3	0	10	-.028	3	0	2	0	1	-.003	4
585	8	max	0	10	.448	4	.031	1	0	3	0	3	0	10
586		min	-62.094	3	0	10	-.028	3	0	2	0	1	-.003	4
587	9	max	0	10	.224	4	.031	1	0	3	0	3	0	10
588		min	-62.029	3	0	10	-.028	3	0	2	0	1	-.003	4
589	10	max	0	10	0	1	.031	1	0	3	0	3	0	10
590		min	-61.964	3	0	1	-.028	3	0	2	0	1	-.003	4
591	11	max	0	10	0	10	.031	1	0	3	0	3	0	10
592		min	-61.898	3	-.224	4	-.028	3	0	2	0	1	-.003	4
593	12	max	0	10	0	10	.031	1	0	3	0	3	0	10
594		min	-61.833	3	-.448	4	-.028	3	0	2	0	1	-.003	4
595	13	max	0	10	0	10	.031	1	0	3	0	2	0	10
596		min	-61.768	3	-.672	4	-.028	3	0	2	0	4	-.003	4
597	14	max	.011	2	0	10	.031	1	0	3	0	2	0	10
598		min	-61.703	3	-.896	4	-.028	3	0	2	0	3	-.003	4
599	15	max	.098	2	0	10	.031	1	0	3	0	1	0	10
600		min	-61.638	3	-1.12	4	-.028	3	0	2	0	3	-.002	4
601	16	max	.185	2	0	10	.031	1	0	3	0	1	0	10
602		min	-61.572	3	-1.344	4	-.028	3	0	2	0	3	-.002	4
603	17	max	.272	2	0	10	.031	1	0	3	0	1	0	10
604		min	-61.507	3	-1.568	4	-.028	3	0	2	0	3	-.001	4
605	18	max	.359	2	0	10	.031	1	0	3	0	1	0	10
606		min	-61.442	3	-1.792	4	-.028	3	0	2	0	3	0	4
607	19	max	.446	2	0	10	.031	1	0	3	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-61.377	3	-2.016	4	-.028	3	0	2	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.009	2	.008	1	-2.352e-5	15	NC	3	NC	2
2			min	-.003	3	-.008	3	-.001	3	-6.235e-4	1	4105.946	2	4392.654	1
3		2	max	.002	1	.008	2	.008	1	-2.253e-5	15	NC	3	NC	2
4			min	-.003	3	-.008	3	-.001	3	-5.976e-4	1	4460.241	2	4746.502	1
5		3	max	.002	1	.007	2	.007	1	-2.155e-5	15	NC	3	NC	2
6			min	-.003	3	-.008	3	-.001	3	-5.717e-4	1	4877.905	2	5163.797	1
7		4	max	.002	1	.007	2	.006	1	-2.057e-5	15	NC	3	NC	2
8			min	-.003	3	-.007	3	0	3	-5.457e-4	1	5373.53	2	5659.893	1
9		5	max	.002	1	.006	2	.006	1	-1.958e-5	15	NC	1	NC	2
10			min	-.003	3	-.007	3	0	3	-5.198e-4	1	5966.366	2	6255.297	1
11		6	max	.002	1	.005	2	.005	1	-1.86e-5	15	NC	1	NC	2
12			min	-.002	3	-.007	3	0	3	-4.939e-4	1	6682.212	2	6977.894	1
13		7	max	.002	1	.005	2	.005	1	-1.762e-5	15	NC	1	NC	2
14			min	-.002	3	-.006	3	0	3	-4.68e-4	1	7556.29	2	7866.404	1
15		8	max	.002	1	.004	2	.004	1	-1.664e-5	15	NC	1	NC	2
16			min	-.002	3	-.006	3	0	3	-4.42e-4	1	8637.739	2	8975.889	1
17		9	max	.001	1	.004	2	.004	1	-1.565e-5	15	NC	1	NC	1
18			min	-.002	3	-.005	3	0	3	-4.161e-4	1	9996.874	2	NC	1
19		10	max	.001	1	.003	2	.003	1	-1.467e-5	15	NC	1	NC	1
20			min	-.002	3	-.005	3	0	3	-3.902e-4	1	NC	1	NC	1
21		11	max	.001	1	.003	2	.002	1	-1.369e-5	15	NC	1	NC	1
22			min	-.001	3	-.004	3	0	3	-3.643e-4	1	NC	1	NC	1
23		12	max	0	1	.002	2	.002	1	-1.27e-5	15	NC	1	NC	1
24			min	-.001	3	-.004	3	0	3	-3.383e-4	1	NC	1	NC	1
25		13	max	0	1	.002	2	.002	1	-1.172e-5	15	NC	1	NC	1
26			min	-.001	3	-.003	3	0	3	-3.124e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	.001	1	-1.074e-5	15	NC	1	NC	1
28			min	0	3	-.003	3	0	3	-2.865e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	0	1	-9.753e-6	15	NC	1	NC	1
30			min	0	3	-.002	3	0	3	-2.606e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	0	1	-8.769e-6	15	NC	1	NC	1
32			min	0	3	-.002	3	0	3	-2.346e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	-7.786e-6	15	NC	1	NC	1
34			min	0	3	-.001	3	0	3	-2.087e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	-6.803e-6	15	NC	1	NC	1
36			min	0	3	0	3	0	3	-1.828e-4	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	-5.82e-6	15	NC	1	NC	1
38			min	0	1	0	1	0	1	-1.569e-4	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	7.296e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	2.707e-6	15	NC	1	NC	1
41		2	max	0	3	0	2	0	12	9.026e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	3.355e-6	15	NC	1	NC	1
43		3	max	0	3	0	2	0	12	1.076e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	4.003e-6	15	NC	1	NC	1
45		4	max	0	3	0	2	0	12	1.248e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	4.65e-6	15	NC	1	NC	1
47		5	max	0	3	0	2	0	3	1.421e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	5.298e-6	15	NC	1	NC	1
49		6	max	0	3	0	2	0	3	1.594e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	5.946e-6	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	1.767e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	1	6.593e-6	15	NC	1	NC	1
53	8	max	0	3	.001	2	0	3	1.94e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	7.241e-6	15	NC	1	NC	1
55	9	max	0	3	.002	2	0	3	2.113e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	1	7.888e-6	15	NC	1	NC	1
57	10	max	0	3	.002	2	0	2	2.286e-4	1	NC	1	NC	1
58		min	0	2	-.007	3	0	15	8.536e-6	15	NC	1	NC	1
59	11	max	0	3	.003	2	0	1	2.459e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	15	9.184e-6	15	NC	1	NC	1
61	12	max	0	3	.003	2	0	1	2.632e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	9.831e-6	15	NC	1	NC	1
63	13	max	0	3	.004	2	.001	1	2.805e-4	1	NC	1	NC	1
64		min	0	2	-.008	3	0	15	1.048e-5	15	NC	1	NC	1
65	14	max	.001	3	.005	2	.002	1	2.978e-4	1	NC	1	NC	1
66		min	-.001	2	-.008	3	0	15	1.113e-5	15	9624.879	2	NC	1
67	15	max	.001	3	.006	2	.002	1	3.151e-4	1	NC	1	NC	1
68		min	-.001	2	-.008	3	0	15	1.177e-5	15	8123.978	2	NC	1
69	16	max	.001	3	.007	2	.003	1	3.324e-4	1	NC	1	NC	1
70		min	-.001	2	-.008	3	0	15	1.242e-5	15	6954.635	2	NC	1
71	17	max	.001	3	.008	2	.003	1	3.497e-4	1	NC	3	NC	1
72		min	-.001	2	-.008	3	0	15	1.307e-5	15	6034.719	2	NC	1
73	18	max	.001	3	.009	2	.003	1	3.669e-4	1	NC	3	NC	1
74		min	-.001	2	-.008	3	0	15	1.372e-5	15	5304.913	2	NC	1
75	19	max	.001	3	.01	2	.004	1	3.842e-4	1	NC	3	NC	1
76		min	-.001	2	-.008	3	0	15	1.436e-5	15	4722.106	2	NC	1
77	M4	1	max	.002	1	.01	2	15	-1.929e-5	15	NC	1	NC	2
78		min	0	3	-.008	3	-.003	1	-5.21e-4	1	NC	1	6748.633	1
79	2	max	.002	1	.01	2	0	15	-1.929e-5	15	NC	1	NC	2
80		min	0	3	-.008	3	-.003	1	-5.21e-4	1	NC	1	7360.962	1
81	3	max	.002	1	.009	2	0	15	-1.929e-5	15	NC	1	NC	2
82		min	0	3	-.007	3	-.002	1	-5.21e-4	1	NC	1	8089.789	1
83	4	max	.002	1	.008	2	0	15	-1.929e-5	15	NC	1	NC	2
84		min	0	3	-.007	3	-.002	1	-5.21e-4	1	NC	1	8965.829	1
85	5	max	.001	1	.008	2	0	15	-1.929e-5	15	NC	1	NC	1
86		min	0	3	-.006	3	-.002	1	-5.21e-4	1	NC	1	NC	1
87	6	max	.001	1	.007	2	0	15	-1.929e-5	15	NC	1	NC	1
88		min	0	3	-.006	3	-.002	1	-5.21e-4	1	NC	1	NC	1
89	7	max	.001	1	.007	2	0	15	-1.929e-5	15	NC	1	NC	1
90		min	0	3	-.006	3	-.001	1	-5.21e-4	1	NC	1	NC	1
91	8	max	.001	1	.006	2	0	15	-1.929e-5	15	NC	1	NC	1
92		min	0	3	-.005	3	-.001	1	-5.21e-4	1	NC	1	NC	1
93	9	max	.001	1	.006	2	0	15	-1.929e-5	15	NC	1	NC	1
94		min	0	3	-.005	3	-.001	1	-5.21e-4	1	NC	1	NC	1
95	10	max	0	1	.005	2	0	15	-1.929e-5	15	NC	1	NC	1
96		min	0	3	-.004	3	0	1	-5.21e-4	1	NC	1	NC	1
97	11	max	0	1	.004	2	0	15	-1.929e-5	15	NC	1	NC	1
98		min	0	3	-.004	3	0	1	-5.21e-4	1	NC	1	NC	1
99	12	max	0	1	.004	2	0	15	-1.929e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	0	1	-5.21e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-1.929e-5	15	NC	1	NC	1
102		min	0	3	-.003	3	0	1	-5.21e-4	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-1.929e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-5.21e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-1.929e-5	15	NC	1	NC	1
106		min	0	3	-.002	3	0	1	-5.21e-4	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-1.929e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-5.21e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-1.929e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-5.21e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.929e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-5.21e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.929e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-5.21e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.026	2	.003	1	3.141e-4	3	NC	3	NC	1
116			min	-.01	3	-.023	3	-.004	3	-6.189e-8	10	1378.654	2	8959.052	3
117		2	max	.007	1	.025	2	.003	1	3.05e-4	3	NC	3	NC	1
118			min	-.01	3	-.022	3	-.004	3	-5.849e-8	10	1474.358	2	9561.844	3
119		3	max	.007	1	.023	2	.003	1	2.96e-4	3	NC	3	NC	1
120			min	-.009	3	-.02	3	-.004	3	-1.238e-6	2	1583.921	2	NC	1
121		4	max	.007	1	.021	2	.003	1	2.869e-4	3	NC	3	NC	1
122			min	-.009	3	-.019	3	-.003	3	-2.78e-6	2	1710.125	2	NC	1
123		5	max	.006	1	.02	2	.002	1	2.778e-4	3	NC	3	NC	1
124			min	-.008	3	-.018	3	-.003	3	-5.314e-6	1	1856.555	2	NC	1
125		6	max	.006	1	.018	2	.002	1	2.688e-4	3	NC	3	NC	1
126			min	-.008	3	-.017	3	-.003	3	-1.002e-5	1	2027.906	2	NC	1
127		7	max	.005	1	.016	2	.002	1	2.597e-4	3	NC	3	NC	1
128			min	-.007	3	-.016	3	-.002	3	-1.473e-5	1	2230.442	2	NC	1
129		8	max	.005	1	.015	2	.002	1	2.506e-4	3	NC	3	NC	1
130			min	-.006	3	-.014	3	-.002	3	-1.944e-5	1	2472.715	2	NC	1
131		9	max	.004	1	.013	2	.001	1	2.415e-4	3	NC	3	NC	1
132			min	-.006	3	-.013	3	-.002	3	-2.415e-5	1	2766.699	2	NC	1
133		10	max	.004	1	.012	2	.001	1	2.325e-4	3	NC	3	NC	1
134			min	-.005	3	-.012	3	-.002	3	-2.886e-5	1	3129.697	2	NC	1
135		11	max	.004	1	.01	2	.001	1	2.234e-4	3	NC	3	NC	1
136			min	-.005	3	-.011	3	-.001	3	-3.357e-5	1	3587.663	2	NC	1
137		12	max	.003	1	.009	2	0	1	2.143e-4	3	NC	3	NC	1
138			min	-.004	3	-.009	3	-.001	3	-3.828e-5	1	4181.383	2	NC	1
139		13	max	.003	1	.007	2	0	1	2.052e-4	3	NC	3	NC	1
140			min	-.003	3	-.008	3	0	3	-4.299e-5	1	4978.831	2	NC	1
141		14	max	.002	1	.006	2	0	1	1.962e-4	3	NC	3	NC	1
142			min	-.003	3	-.007	3	0	3	-4.77e-5	1	6102.347	2	NC	1
143		15	max	.002	1	.005	2	0	1	1.871e-4	3	NC	1	NC	1
144			min	-.002	3	-.005	3	0	3	-5.241e-5	1	7796.59	2	NC	1
145		16	max	.001	1	.003	2	0	1	1.78e-4	3	NC	1	NC	1
146			min	-.002	3	-.004	3	0	3	-5.711e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.689e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-6.182e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.599e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-6.653e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.508e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-7.124e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.285e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-6.984e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	2.792e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-5.274e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.298e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-3.565e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.805e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.855e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	.001	3	1.311e-5	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-1.455e-6	3	9234.078	2	NC	1
163		6	max	.001	3	.006	2	.001	3	1.564e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	1	0	10	7407.832	2	NC	1
165		7	max	.001	3	.007	2	.002	3	3.274e-5	3	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.01	3	0	1	0	10	6156.463	2	NC	1
167		8	max	.002	3	.009	2	.002	3	4.983e-5	3	NC	3	NC	1
168			min	-.002	2	-.011	3	-.001	1	-1.689e-6	1	5237.179	2	NC	1
169		9	max	.002	3	.01	2	.002	3	6.693e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	-.001	1	-6.623e-6	1	4529.106	2	NC	1
171		10	max	.002	3	.012	2	.002	3	8.402e-5	3	NC	3	NC	1
172			min	-.003	2	-.014	3	-.001	1	-1.156e-5	1	3965.295	2	NC	1
173		11	max	.002	3	.013	2	.002	3	1.011e-4	3	NC	3	NC	1
174			min	-.003	2	-.015	3	-.001	1	-1.649e-5	1	3505.554	2	NC	1
175		12	max	.003	3	.015	2	.002	3	1.182e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-2.143e-5	1	3124.149	2	NC	1
177		13	max	.003	3	.016	2	.002	3	1.353e-4	3	NC	3	NC	1
178			min	-.004	2	-.018	3	-.002	1	-2.636e-5	1	2803.695	2	NC	1
179		14	max	.003	3	.018	2	.002	3	1.524e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	-.002	1	-3.129e-5	1	2531.907	2	NC	1
181		15	max	.003	3	.02	2	.002	3	1.695e-4	3	NC	3	NC	1
182			min	-.004	2	-.019	3	-.002	1	-3.623e-5	1	2299.77	2	NC	1
183		16	max	.004	3	.022	2	.002	3	1.866e-4	3	NC	3	NC	1
184			min	-.004	2	-.02	3	-.002	1	-4.116e-5	1	2100.462	2	NC	1
185		17	max	.004	3	.024	2	.002	3	2.037e-4	3	NC	3	NC	1
186			min	-.005	2	-.021	3	-.002	1	-4.61e-5	1	1928.694	2	NC	1
187		18	max	.004	3	.026	2	.002	3	2.208e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	-.002	1	-5.103e-5	1	1780.282	2	NC	1
189		19	max	.004	3	.028	2	.002	3	2.379e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	-.002	1	-5.597e-5	1	1651.873	2	NC	1
191	M8	1	max	.005	1	.03	2	.002	1	-8.484e-8	10	NC	1	NC	1
192			min	0	3	-.023	3	-.001	3	-1.81e-4	3	NC	1	NC	1
193		2	max	.005	1	.028	2	.002	1	-8.484e-8	10	NC	1	NC	1
194			min	0	3	-.022	3	-.001	3	-1.81e-4	3	NC	1	NC	1
195		3	max	.005	1	.027	2	.002	1	-8.484e-8	10	NC	1	NC	1
196			min	0	3	-.02	3	-.001	3	-1.81e-4	3	NC	1	NC	1
197		4	max	.004	1	.025	2	.001	1	-8.484e-8	10	NC	1	NC	1
198			min	0	3	-.019	3	-.001	3	-1.81e-4	3	NC	1	NC	1
199		5	max	.004	1	.023	2	.001	1	-8.484e-8	10	NC	1	NC	1
200			min	0	3	-.018	3	0	3	-1.81e-4	3	NC	1	NC	1
201		6	max	.004	1	.022	2	.001	1	-8.484e-8	10	NC	1	NC	1
202			min	0	3	-.016	3	0	3	-1.81e-4	3	NC	1	NC	1
203		7	max	.003	1	.02	2	0	1	-8.484e-8	10	NC	1	NC	1
204			min	0	3	-.015	3	0	3	-1.81e-4	3	NC	1	NC	1
205		8	max	.003	1	.018	2	0	1	-8.484e-8	10	NC	1	NC	1
206			min	0	3	-.014	3	0	3	-1.81e-4	3	NC	1	NC	1
207		9	max	.003	1	.017	2	0	1	-8.484e-8	10	NC	1	NC	1
208			min	0	3	-.013	3	0	3	-1.81e-4	3	NC	1	NC	1
209		10	max	.003	1	.015	2	0	1	-8.484e-8	10	NC	1	NC	1
210			min	0	3	-.011	3	0	3	-1.81e-4	3	NC	1	NC	1
211		11	max	.002	1	.013	2	0	1	-8.484e-8	10	NC	1	NC	1
212			min	0	3	-.01	3	0	3	-1.81e-4	3	NC	1	NC	1
213		12	max	.002	1	.012	2	0	1	-8.484e-8	10	NC	1	NC	1
214			min	0	3	-.009	3	0	3	-1.81e-4	3	NC	1	NC	1
215		13	max	.002	1	.01	2	0	1	-8.484e-8	10	NC	1	NC	1
216			min	0	3	-.008	3	0	3	-1.81e-4	3	NC	1	NC	1
217		14	max	.001	1	.008	2	0	1	-8.484e-8	10	NC	1	NC	1
218			min	0	3	-.006	3	0	3	-1.81e-4	3	NC	1	NC	1
219		15	max	.001	1	.007	2	0	1	-8.484e-8	10	NC	1	NC	1
220			min	0	3	-.005	3	0	3	-1.81e-4	3	NC	1	NC	1
221		16	max	0	1	.005	2	0	1	-8.484e-8	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-1.81e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-8.484e-8	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-1.81e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-8.484e-8	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.81e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-8.484e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.81e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.009	2	0	3	6.436e-4	1	NC	3	NC	1
230			min	-.003	3	-.008	3	-.001	1	-3.634e-4	3	4110.754	2	NC	1
231		2	max	.002	1	.008	2	0	3	6.105e-4	1	NC	3	NC	1
232			min	-.003	3	-.008	3	-.001	1	-3.517e-4	3	4465.585	2	NC	1
233		3	max	.002	1	.007	2	0	3	5.773e-4	1	NC	3	NC	1
234			min	-.003	3	-.008	3	-.001	1	-3.4e-4	3	4883.903	2	NC	1
235		4	max	.002	1	.007	2	0	3	5.442e-4	1	NC	3	NC	1
236			min	-.003	3	-.007	3	-.001	1	-3.284e-4	3	5380.332	2	NC	1
237		5	max	.002	1	.006	2	0	3	5.11e-4	1	NC	1	NC	1
238			min	-.002	3	-.007	3	-.001	1	-3.167e-4	3	5974.166	2	NC	1
239		6	max	.002	1	.005	2	0	3	4.779e-4	1	NC	1	NC	1
240			min	-.002	3	-.007	3	-.001	1	-3.05e-4	3	6691.263	2	NC	1
241		7	max	.002	1	.005	2	0	3	4.447e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	0	1	-2.933e-4	3	7566.931	2	NC	1
243		8	max	.002	1	.004	2	0	3	4.116e-4	1	NC	1	NC	1
244			min	-.002	3	-.006	3	0	1	-2.816e-4	3	8650.429	2	NC	1
245		9	max	.001	1	.004	2	0	3	3.784e-4	1	NC	1	NC	1
246			min	-.002	3	-.005	3	0	1	-2.699e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	3.453e-4	1	NC	1	NC	1
248			min	-.002	3	-.005	3	0	1	-2.582e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	3.121e-4	1	NC	1	NC	1
250			min	-.001	3	-.005	3	0	1	-2.465e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	2.789e-4	1	NC	1	NC	1
252			min	-.001	3	-.004	3	0	1	-2.348e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	2.458e-4	1	NC	1	NC	1
254			min	-.001	3	-.004	3	0	1	-2.231e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	2.126e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-2.114e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.795e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-1.997e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.463e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.88e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.132e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-1.763e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	8.002e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.646e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	4.686e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.529e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.124e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.244e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	5.358e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-5.016e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	3.593e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-7.788e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	1.828e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.056e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	6.249e-7	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-1.333e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-6.467e-6	15	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-1.61e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-7.552e-6	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-1.888e-4	1	NC	1	NC	1
281		8	max	0	3	.001	2	0	10	-8.637e-6	15	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-2.165e-4	1	NC	1	NC	1
283		9	max	0	3	.002	2	0	10	-9.721e-6	15	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-2.442e-4	1	NC	1	NC	1
285		10	max	0	3	.002	2	0	15	-1.081e-5	15	NC	1	NC	1
286			min	0	2	-.007	3	-.002	1	-2.719e-4	1	NC	1	NC	1
287		11	max	0	3	.003	2	0	15	-1.189e-5	15	NC	1	NC	1
288			min	0	2	-.007	3	-.003	1	-2.996e-4	1	NC	1	NC	1
289		12	max	0	3	.003	2	0	15	-1.298e-5	15	NC	1	NC	1
290			min	0	2	-.008	3	-.004	1	-3.273e-4	1	NC	1	NC	1
291		13	max	0	3	.004	2	0	15	-1.406e-5	15	NC	1	NC	1
292			min	0	2	-.008	3	-.004	1	-3.551e-4	1	NC	1	NC	1
293		14	max	.001	3	.005	2	0	15	-1.515e-5	15	NC	1	NC	2
294			min	-.001	2	-.008	3	-.005	1	-3.828e-4	1	9640.454	2	9385.562	1
295		15	max	.001	3	.006	2	0	15	-1.623e-5	15	NC	1	NC	2
296			min	-.001	2	-.008	3	-.006	1	-4.105e-4	1	8135.856	2	8319.457	1
297		16	max	.001	3	.007	2	0	15	-1.732e-5	15	NC	1	NC	2
298			min	-.001	2	-.008	3	-.006	1	-4.382e-4	1	6963.91	2	7478.029	1
299		17	max	.001	3	.008	2	0	15	-1.84e-5	15	NC	3	NC	2
300			min	-.001	2	-.008	3	-.007	1	-4.659e-4	1	6042.13	2	6805.51	1
301		18	max	.001	3	.009	2	0	15	-1.949e-5	15	NC	3	NC	2
302			min	-.001	2	-.008	3	-.007	1	-4.937e-4	1	5310.969	2	6263.21	1
303		19	max	.001	3	.01	2	0	15	-2.057e-5	15	NC	3	NC	2
304			min	-.001	2	-.008	3	-.008	1	-5.214e-4	1	4727.165	2	5823.713	1
305	M12	1	max	.002	1	.01	2	.007	1	4.57e-4	1	NC	1	NC	3
306			min	0	3	-.008	3	0	15	1.799e-5	15	NC	1	2884.314	1
307		2	max	.002	1	.01	2	.006	1	4.57e-4	1	NC	1	NC	2
308			min	0	3	-.008	3	0	15	1.799e-5	15	NC	1	3145.272	1
309		3	max	.002	1	.009	2	.006	1	4.57e-4	1	NC	1	NC	2
310			min	0	3	-.007	3	0	15	1.799e-5	15	NC	1	3455.914	1
311		4	max	.002	1	.008	2	.005	1	4.57e-4	1	NC	1	NC	2
312			min	0	3	-.007	3	0	15	1.799e-5	15	NC	1	3829.333	1
313		5	max	.001	1	.008	2	.005	1	4.57e-4	1	NC	1	NC	2
314			min	0	3	-.007	3	0	15	1.799e-5	15	NC	1	4283.366	1
315		6	max	.001	1	.007	2	.004	1	4.57e-4	1	NC	1	NC	2
316			min	0	3	-.006	3	0	15	1.799e-5	15	NC	1	4842.823	1
317		7	max	.001	1	.007	2	.003	1	4.57e-4	1	NC	1	NC	2
318			min	0	3	-.006	3	0	15	1.799e-5	15	NC	1	5543.042	1
319		8	max	.001	1	.006	2	.003	1	4.57e-4	1	NC	1	NC	2
320			min	0	3	-.005	3	0	15	1.799e-5	15	NC	1	6435.773	1
321		9	max	.001	1	.006	2	.003	1	4.57e-4	1	NC	1	NC	2
322			min	0	3	-.005	3	0	15	1.799e-5	15	NC	1	7599.294	1
323		10	max	0	1	.005	2	.002	1	4.57e-4	1	NC	1	NC	2
324			min	0	3	-.004	3	0	15	1.799e-5	15	NC	1	9156.673	1
325		11	max	0	1	.004	2	.002	1	4.57e-4	1	NC	1	NC	1
326			min	0	3	-.004	3	0	15	1.799e-5	15	NC	1	NC	1
327		12	max	0	1	.004	2	.001	1	4.57e-4	1	NC	1	NC	1
328			min	0	3	-.003	3	0	15	1.799e-5	15	NC	1	NC	1
329		13	max	0	1	.003	2	.001	1	4.57e-4	1	NC	1	NC	1
330			min	0	3	-.003	3	0	15	1.799e-5	15	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	4.57e-4	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	1.799e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	4.57e-4	1	NC	1	NC	1
334			min	0	3	-.002	3	0	15	1.799e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	4.57e-4	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	1.799e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	4.57e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	1.799e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.57e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	1.799e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.57e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.799e-5	15	NC	1	NC	1
343	M1	1	max	.008	3	.025	3	.002	3	6.889e-3	1	NC	1	NC	1
344			min	-.008	2	-.023	2	-.003	1	-8.488e-3	3	NC	1	NC	1
345		2	max	.008	3	.014	3	.002	3	3.217e-3	1	NC	4	NC	1
346			min	-.008	2	-.013	2	-.006	1	-4.188e-3	3	4372.756	2	NC	1
347		3	max	.008	3	.004	3	.001	3	3.316e-5	3	NC	4	NC	2
348			min	-.008	2	-.003	2	-.008	1	-3.868e-4	1	2252.651	2	9100.205	1
349		4	max	.008	3	.005	2	0	3	3.462e-5	3	NC	4	NC	2
350			min	-.008	2	-.004	3	-.009	1	-3.29e-4	1	1579.642	2	7546.635	1
351		5	max	.008	3	.013	2	0	3	3.608e-5	3	NC	5	NC	2
352			min	-.008	2	-.011	3	-.009	1	-2.713e-4	1	1254.147	2	7268.427	1
353		6	max	.008	3	.019	2	0	3	3.755e-5	3	NC	5	NC	2
354			min	-.008	2	-.017	3	-.009	1	-2.136e-4	1	1068.476	2	7812.195	1
355		7	max	.008	3	.024	2	0	3	3.901e-5	3	NC	5	NC	2
356			min	-.008	2	-.021	3	-.008	1	-1.558e-4	1	954.316	2	9370.394	1
357		8	max	.008	3	.027	2	0	3	4.047e-5	3	NC	5	NC	1
358			min	-.008	2	-.024	3	-.006	1	-9.812e-5	1	883.131	2	NC	1
359		9	max	.008	3	.03	2	0	3	4.194e-5	3	NC	5	NC	1
360			min	-.008	2	-.026	3	-.005	1	-4.039e-5	1	841.472	2	NC	1
361		10	max	.008	3	.031	2	0	3	4.34e-5	3	NC	5	NC	1
362			min	-.008	2	-.026	3	-.003	1	7.236e-7	15	822.958	2	NC	1
363		11	max	.008	3	.03	2	0	3	7.507e-5	1	NC	5	NC	1
364			min	-.008	2	-.025	3	0	1	2.885e-6	15	825.348	2	NC	1
365		12	max	.008	3	.028	2	0	1	1.328e-4	1	NC	5	NC	1
366			min	-.008	2	-.023	3	0	15	5.046e-6	15	849.693	2	NC	1
367		13	max	.008	3	.025	2	.002	1	1.905e-4	1	NC	5	NC	2
368			min	-.009	2	-.02	3	0	15	7.207e-6	15	900.873	2	9651.221	1
369		14	max	.008	3	.02	2	.003	1	2.483e-4	1	NC	5	NC	2
370			min	-.009	2	-.015	3	0	15	9.369e-6	15	990.037	2	7985.621	1
371		15	max	.008	3	.013	2	.004	1	3.06e-4	1	NC	5	NC	2
372			min	-.009	2	-.01	3	0	15	1.153e-5	15	1141.54	2	7395.371	1
373		16	max	.008	3	.005	2	.004	1	3.469e-4	1	NC	4	NC	2
374			min	-.009	2	-.004	3	0	15	1.307e-5	15	1414.625	2	7650.027	1
375		17	max	.008	3	.004	3	.003	1	4.105e-5	3	NC	4	NC	2
376			min	-.009	2	-.005	2	0	15	-1.297e-5	1	1996.329	2	9202.711	1
377		18	max	.008	3	.012	3	0	1	4.334e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	15	-1.969e-3	3	3852.882	2	NC	1
379		19	max	.008	3	.02	3	0	3	8.729e-3	2	NC	1	NC	1
380			min	-.009	2	-.03	2	-.002	1	-4.016e-3	3	NC	1	NC	1
381	M5	1	max	.021	3	.071	3	.002	3	2.276e-6	3	NC	1	NC	1
382			min	-.025	2	-.065	2	-.004	1	3.427e-8	2	NC	1	NC	1
383		2	max	.021	3	.041	3	.003	3	8.54e-5	3	NC	4	NC	1
384			min	-.025	2	-.037	2	-.004	1	-6.538e-5	1	1597.916	2	NC	1
385		3	max	.021	3	.013	3	.004	3	1.669e-4	3	NC	5	NC	1
386			min	-.025	2	-.01	1	-.003	1	-1.296e-4	1	818.586	2	NC	1
387		4	max	.021	3	.014	2	.005	3	1.625e-4	3	NC	5	NC	1
388			min	-.025	2	-.01	3	-.003	1	-1.234e-4	1	572.579	2	NC	1
389		5	max	.021	3	.034	2	.005	3	1.581e-4	3	NC	5	NC	1
390			min	-.025	2	-.03	3	-.003	1	-1.172e-4	1	453.631	2	NC	1
391		6	max	.021	3	.051	2	.005	3	1.537e-4	3	NC	5	NC	1
392			min	-.025	2	-.045	3	-.003	1	-1.109e-4	1	385.737	2	NC	1
393		7	max	.021	3	.065	2	.006	3	1.492e-4	3	NC	5	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.008	3	.004	3	0	15	2.014e-5	3	NC	4	NC	2
452			min	-.009	2	-.005	2	-.007	1	-2.784e-4	1	1997.144	2	8899.453	1
453		18	max	.008	3	.012	3	0	15	2.e-3	3	NC	4	NC	1
454			min	-.009	2	-.017	2	-.005	1	-4.36e-3	2	3854.393	2	NC	1
455		19	max	.008	3	.02	3	0	3	4.015e-3	3	NC	1	NC	1
456			min	-.009	2	-.03	2	-.001	1	-8.729e-3	2	NC	1	NC	1
457	M13	1	max	.004	1	.025	3	.008	3	4.167e-3	3	NC	1	NC	1
458			min	-.002	3	-.023	2	-.008	2	-3.961e-3	2	NC	1	NC	1
459		2	max	.004	1	.088	3	.012	1	4.961e-3	3	NC	4	NC	2
460			min	-.002	3	-.074	1	-.004	10	-4.708e-3	2	2194.81	3	8177.808	1
461		3	max	.004	1	.14	3	.033	1	5.755e-3	3	NC	5	NC	2
462			min	-.002	3	-.116	1	-.002	10	-5.455e-3	2	1199.358	3	3643.17	1
463		4	max	.004	1	.174	3	.049	1	6.55e-3	3	NC	5	NC	3
464			min	-.002	3	-.144	1	-.002	10	-6.203e-3	2	924.235	3	2531.292	1
465		5	max	.004	1	.187	3	.056	1	7.344e-3	3	NC	5	NC	3
466			min	-.002	3	-.155	1	-.002	10	-6.95e-3	2	851.103	3	2244.814	1
467		6	max	.004	1	.179	3	.052	1	8.138e-3	3	NC	5	NC	3
468			min	-.002	3	-.15	1	-.004	10	-7.698e-3	2	897.137	3	2437.497	1
469		7	max	.004	1	.154	3	.036	1	8.932e-3	3	NC	5	NC	2
470			min	-.002	3	-.13	1	-.007	10	-8.445e-3	2	1074.906	3	3367.305	1
471		8	max	.004	1	.119	3	.017	3	9.727e-3	3	NC	5	NC	2
472			min	-.002	3	-.103	1	-.011	2	-9.192e-3	2	1474.146	3	7340.951	1
473		9	max	.004	1	.086	3	.019	3	1.052e-2	3	NC	4	NC	1
474			min	-.002	3	-.077	1	-.021	2	-9.94e-3	2	2260.063	3	NC	1
475		10	max	.004	1	.071	3	.021	3	1.132e-2	3	NC	4	NC	1
476			min	-.002	3	-.065	2	-.025	2	-1.069e-2	2	2998.09	3	8280.897	2
477		11	max	.004	1	.086	3	.023	3	1.052e-2	3	NC	4	NC	1
478			min	-.002	3	-.077	1	-.021	2	-9.94e-3	2	2260.061	3	8868.128	3
479		12	max	.004	1	.119	3	.025	3	9.729e-3	3	NC	5	NC	2
480			min	-.002	3	-.103	1	-.011	2	-9.193e-3	2	1474.146	3	7238.16	1
481		13	max	.003	1	.154	3	.036	1	8.935e-3	3	NC	5	NC	2
482			min	-.002	3	-.13	1	-.007	10	-8.446e-3	2	1074.905	3	3351.072	1
483		14	max	.003	1	.179	3	.052	1	8.142e-3	3	NC	5	NC	5
484			min	-.002	3	-.15	1	-.004	10	-7.698e-3	2	897.137	3	2434.927	1
485		15	max	.003	1	.187	3	.056	1	7.349e-3	3	NC	5	NC	3
486			min	-.002	3	-.155	1	-.002	10	-6.951e-3	2	851.103	3	2248.509	1
487		16	max	.003	1	.175	3	.049	1	6.555e-3	3	NC	5	NC	3
488			min	-.002	3	-.144	1	-.002	10	-6.204e-3	2	924.234	3	2542.281	1
489		17	max	.003	1	.14	3	.033	1	5.762e-3	3	NC	5	NC	2
490			min	-.002	3	-.116	1	-.002	10	-5.457e-3	2	1199.358	3	3671.848	1
491		18	max	.003	1	.088	3	.012	1	4.969e-3	3	NC	4	NC	2
492			min	-.002	3	-.074	1	-.004	10	-4.71e-3	2	2194.809	3	8292.49	1
493		19	max	.003	1	.025	3	.008	3	4.175e-3	3	NC	1	NC	1
494			min	-.002	3	-.023	2	-.008	2	-3.962e-3	2	NC	1	NC	1
495	M16	1	max	.001	1	.02	3	.008	3	4.774e-3	2	NC	1	NC	1
496			min	0	3	-.03	2	-.009	2	-3.246e-3	3	NC	1	NC	1
497		2	max	.001	1	.05	3	.012	1	5.699e-3	2	NC	4	NC	2
498			min	0	3	-.094	2	-.004	10	-3.823e-3	3	2133.538	2	8188.751	1
499		3	max	.002	1	.075	3	.033	1	6.624e-3	2	NC	5	NC	2
500			min	0	3	-.148	2	-.002	10	-4.401e-3	3	1164.078	2	3647.571	1
501		4	max	.002	1	.092	3	.049	1	7.549e-3	2	NC	5	NC	3
502			min	0	3	-.184	2	-.002	10	-4.979e-3	3	894.577	2	2534.537	1
503		5	max	.002	1	.1	3	.056	1	8.474e-3	2	NC	5	NC	3
504			min	0	3	-.198	2	-.002	10	-5.556e-3	3	820.083	2	2248.293	1
505		6	max	.002	1	.098	3	.051	1	9.399e-3	2	NC	5	NC	5
506			min	0	3	-.19	2	-.004	10	-6.134e-3	3	858.048	2	2442.766	1
507		7	max	.002	1	.088	3	.036	1	1.032e-2	2	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-166	2	-.007	10	-6.712e-3	3	1015.002	2	3379.758	1
509	8	max	.002	1	.074	3	.023	3	1.125e-2	2	NC	5	NC	2
510		min	0	3	-.131	2	-.011	2	-7.289e-3	3	1359.715	2	7412.788	1
511	9	max	.002	1	.06	3	.022	3	1.217e-2	2	NC	4	NC	1
512		min	0	3	-.099	2	-.021	2	-7.867e-3	3	1998.807	2	9709.524	3
513	10	max	.002	1	.054	3	.02	3	1.31e-2	2	NC	4	NC	1
514		min	0	3	-.084	2	-.025	2	-8.445e-3	3	2554.793	2	8276.096	2
515	11	max	.002	1	.06	3	.019	3	1.217e-2	2	NC	4	NC	1
516		min	0	3	-.099	2	-.021	2	-7.866e-3	3	1998.807	2	NC	1
517	12	max	.002	1	.074	3	.019	3	1.125e-2	2	NC	5	NC	2
518		min	0	3	-.131	2	-.011	2	-7.287e-3	3	1359.715	2	7352.92	1
519	13	max	.002	1	.088	3	.036	1	1.032e-2	2	NC	5	NC	2
520		min	0	3	-166	2	-.007	10	-6.709e-3	3	1015.002	2	3375.409	1
521	14	max	.002	1	.098	3	.051	1	9.4e-3	2	NC	5	NC	3
522		min	0	3	-.19	2	-.004	10	-6.13e-3	3	858.048	2	2447.082	1
523	15	max	.002	1	.099	3	.056	1	8.475e-3	2	NC	5	NC	3
524		min	0	3	-.198	2	-.002	10	-5.552e-3	3	820.083	2	2257.773	1
525	16	max	.002	1	.092	3	.049	1	7.55e-3	2	NC	5	NC	3
526		min	0	3	-.184	2	-.002	10	-4.973e-3	3	894.577	2	2552.078	1
527	17	max	.002	1	.075	3	.032	1	6.625e-3	2	NC	5	NC	2
528		min	0	3	-.148	2	-.002	10	-4.395e-3	3	1164.078	2	3686.719	1
529	18	max	.002	1	.05	3	.011	1	5.701e-3	2	NC	4	NC	2
530		min	0	3	-.094	2	-.004	10	-3.816e-3	3	2133.538	2	8333.542	1
531	19	max	.002	1	.02	3	.008	3	4.776e-3	2	NC	1	NC	1
532		min	0	3	-.03	2	-.009	2	-3.238e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	1	3.693e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-8.204e-5	2	NC	1	NC	1
535	2	max	0	3	-.002	15	0	1	8.232e-4	3	NC	1	NC	1
536		min	0	2	-.008	4	0	3	-5.513e-4	2	NC	1	NC	1
537	3	max	0	3	-.004	15	.003	1	1.277e-3	3	NC	5	NC	1
538		min	0	2	-.015	4	-.003	3	-1.021e-3	2	5123.393	4	NC	1
539	4	max	0	3	-.005	15	.006	1	1.731e-3	3	NC	15	NC	4
540		min	0	2	-.022	4	-.006	3	-1.49e-3	2	3514.947	4	7332.907	3
541	5	max	0	3	-.007	15	.01	1	2.185e-3	3	NC	15	NC	4
542		min	0	2	-.029	4	-.011	3	-1.959e-3	2	2742.749	4	4807.413	3
543	6	max	0	3	-.008	15	.015	1	2.639e-3	3	9819.895	15	NC	4
544		min	0	2	-.034	4	-.015	3	-2.428e-3	2	2308.313	4	3497.677	3
545	7	max	0	3	-.009	15	.02	1	3.092e-3	3	8708.475	15	NC	4
546		min	0	2	-.038	4	-.02	3	-2.897e-3	2	2047.057	4	2732.663	3
547	8	max	0	3	-.01	15	.024	1	3.546e-3	3	8041.456	15	NC	4
548		min	-.001	2	-.042	4	-.025	3	-3.367e-3	2	1890.264	4	2252.121	3
549	9	max	0	3	-.01	15	.028	1	4.e-3	3	7682.427	15	NC	4
550		min	-.001	2	-.044	4	-.029	3	-3.836e-3	2	1805.869	4	1937.807	3
551	10	max	0	3	-.01	15	.031	1	4.454e-3	3	7568.847	15	NC	4
552		min	-.001	2	-.044	4	-.033	3	-4.305e-3	2	1779.171	4	1730.392	3
553	11	max	0	3	-.01	15	.034	1	4.908e-3	3	7682.427	15	NC	5
554		min	-.001	2	-.044	4	-.035	3	-4.774e-3	2	1805.869	4	1598.744	3
555	12	max	0	3	-.01	15	.034	1	5.362e-3	3	8041.456	15	NC	5
556		min	-.002	2	-.042	4	-.036	3	-5.244e-3	2	1890.264	4	1527.406	3
557	13	max	0	3	-.009	15	.034	1	5.816e-3	3	8708.475	15	NC	5
558		min	-.002	2	-.039	4	-.035	3	-5.713e-3	2	2047.057	4	1511.85	3
559	14	max	0	3	-.008	15	.031	1	6.269e-3	3	9819.895	15	NC	4
560		min	-.002	2	-.035	4	-.032	3	-6.182e-3	2	2308.313	4	1558.614	3
561	15	max	.001	3	-.007	15	.026	1	6.723e-3	3	NC	15	NC	4
562		min	-.002	2	-.029	4	-.027	3	-6.651e-3	2	2742.749	4	1691.862	3
563	16	max	.001	3	-.005	15	.019	1	7.177e-3	3	NC	15	NC	4
564		min	-.002	2	-.023	4	-.018	3	-7.121e-3	2	3514.947	4	1977.286	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	-.004	15	.009	1	7.631e-3	3	NC	5	NC	4
566		min	-.002	2	-.016	4	-.007	3	-7.59e-3	2	5123.393	4	2621.035	3
567	18	max	.001	3	-.001	12	.007	3	8.085e-3	3	NC	1	NC	4
568		min	-.003	2	-.009	4	-.01	2	-8.059e-3	2	NC	1	4666.018	3
569	19	max	.001	3	.003	3	.025	3	8.539e-3	3	NC	1	NC	1
570		min	-.003	2	-.002	9	-.027	2	-8.528e-3	2	NC	1	NC	1
571	M16A	1	max	0	0	10	.009	3	2.97e-3	3	NC	1	NC	1
572		min	-.001	3	-.001	9	-.009	2	-2.981e-3	2	NC	1	NC	1
573	2	max	0	10	-.002	15	.002	9	2.842e-3	3	NC	1	NC	1
574		min	-.001	3	-.008	4	-.002	2	-2.842e-3	2	NC	1	NC	1
575	3	max	0	10	-.004	15	.008	1	2.715e-3	3	NC	5	NC	4
576		min	-.001	3	-.016	4	-.004	3	-2.703e-3	2	5123.393	4	5836.426	1
577	4	max	0	10	-.005	15	.013	1	2.587e-3	3	NC	15	NC	4
578		min	-.001	3	-.023	4	-.008	3	-2.564e-3	2	3514.947	4	4428.676	1
579	5	max	0	10	-.007	15	.016	1	2.46e-3	3	NC	15	NC	4
580		min	0	3	-.029	4	-.011	3	-2.424e-3	2	2742.749	4	3814.568	1
581	6	max	0	10	-.008	15	.018	1	2.332e-3	3	9819.895	15	NC	4
582		min	0	3	-.034	4	-.013	3	-2.285e-3	2	2308.313	4	3540.927	1
583	7	max	0	10	-.009	15	.018	1	2.205e-3	3	8708.475	15	NC	4
584		min	0	3	-.038	4	-.014	3	-2.146e-3	2	2047.057	4	3465.034	1
585	8	max	0	10	-.01	15	.018	1	2.077e-3	3	8041.456	15	NC	4
586		min	0	3	-.042	4	-.014	3	-2.007e-3	2	1890.264	4	3536.965	1
587	9	max	0	10	-.01	15	.017	1	1.949e-3	3	7682.427	15	NC	4
588		min	0	3	-.043	4	-.013	3	-1.868e-3	2	1805.869	4	3747.796	1
589	10	max	0	10	-.01	15	.016	1	1.822e-3	3	7568.847	15	NC	4
590		min	0	3	-.044	4	-.012	3	-1.729e-3	2	1779.171	4	4116.884	1
591	11	max	0	10	-.01	15	.014	1	1.694e-3	3	7682.427	15	NC	4
592		min	0	3	-.043	4	-.011	3	-1.59e-3	2	1805.869	4	4695.205	1
593	12	max	0	10	-.01	15	.012	1	1.567e-3	3	8041.456	15	NC	4
594		min	0	3	-.041	4	-.009	3	-1.451e-3	2	1890.264	4	5583.918	1
595	13	max	0	10	-.009	15	.009	1	1.439e-3	3	8708.475	15	NC	4
596		min	0	3	-.038	4	-.007	3	-1.311e-3	2	2047.057	4	6981.843	1
597	14	max	0	10	-.008	15	.007	1	1.311e-3	3	9819.895	15	NC	2
598		min	0	3	-.034	4	-.005	3	-1.172e-3	2	2308.313	4	9308.296	1
599	15	max	0	10	-.007	15	.004	1	1.184e-3	3	NC	15	NC	1
600		min	0	3	-.029	4	-.003	3	-1.033e-3	2	2742.749	4	NC	1
601	16	max	0	10	-.005	15	.002	1	1.056e-3	3	NC	15	NC	1
602		min	0	3	-.022	4	-.001	3	-8.941e-4	2	3514.947	4	NC	1
603	17	max	0	10	-.004	15	0	9	9.287e-4	3	NC	5	NC	1
604		min	0	3	-.015	4	0	2	-7.55e-4	2	5123.393	4	NC	1
605	18	max	0	10	-.002	15	0	4	8.011e-4	3	NC	1	NC	1
606		min	0	3	-.008	4	0	2	-6.159e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	6.736e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.767e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

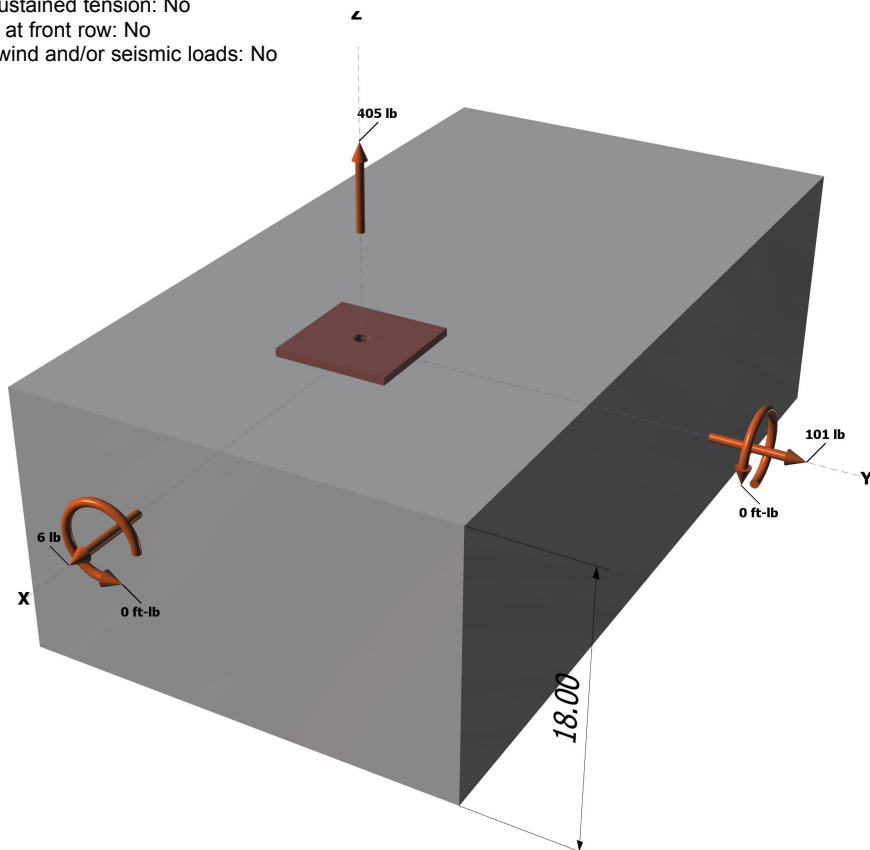
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

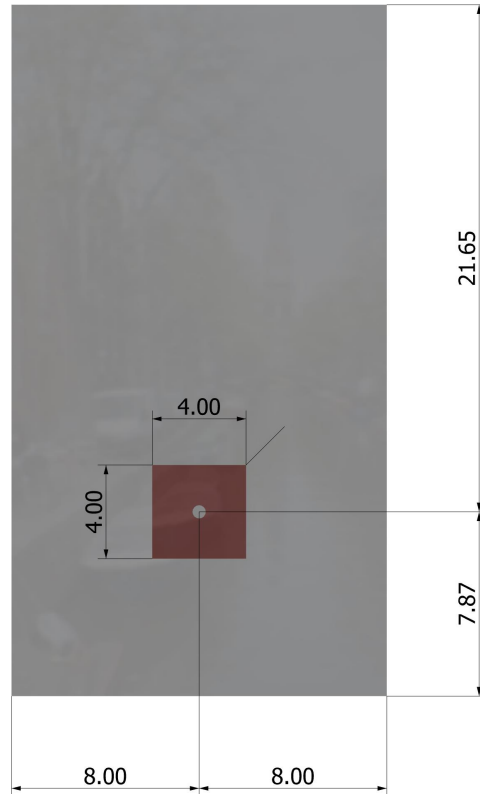
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

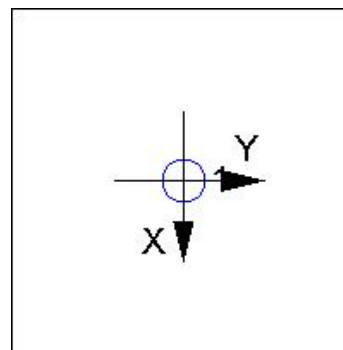
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

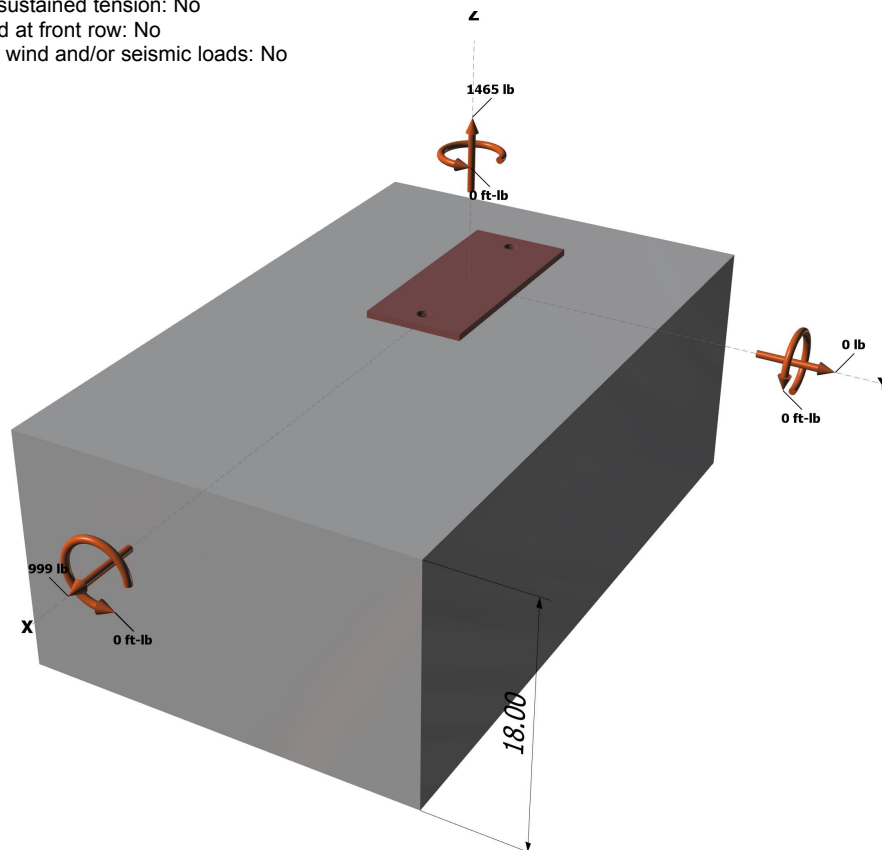
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



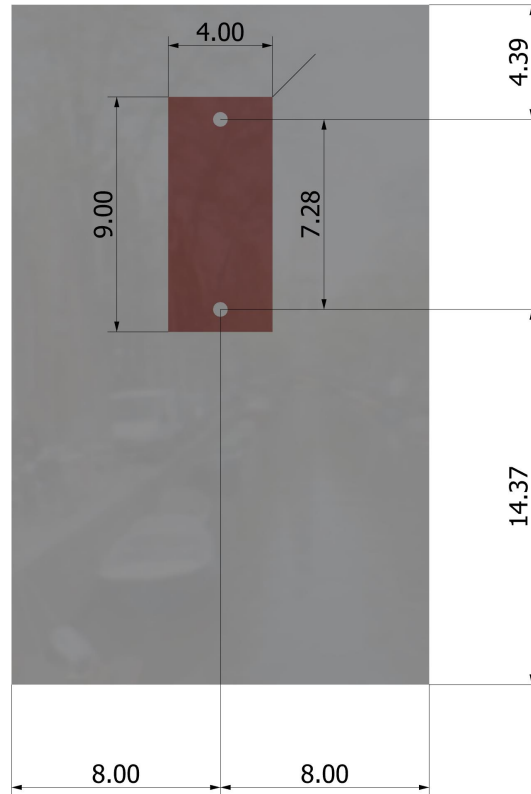
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.