



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

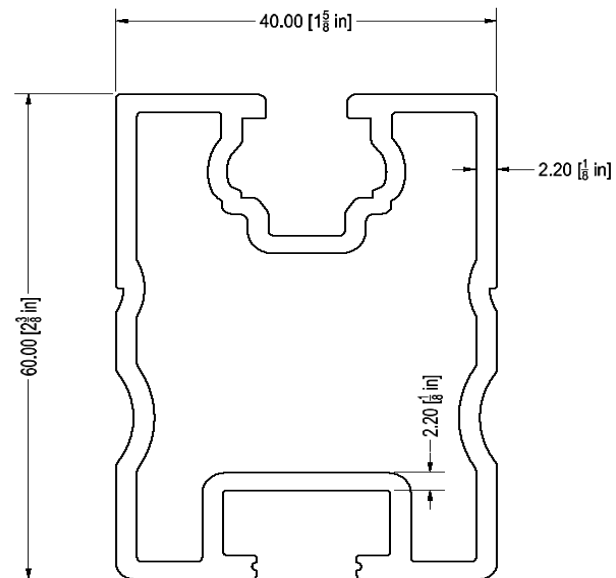
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

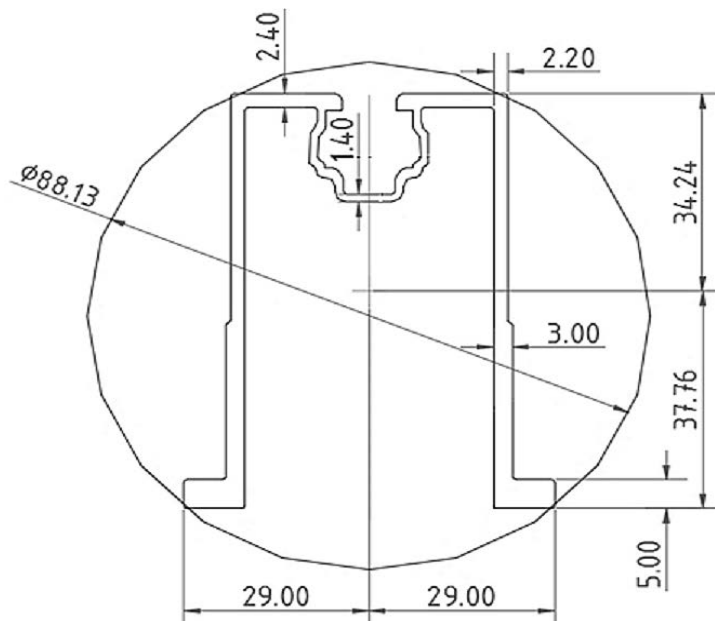
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	63 in
$\Phi F_{ty}$ STRONG-AXIS =	29.20 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.570 k-ft
$M_z$ =	0.053 k-ft
$M_{y \text{ allowable}}$ =	1.243 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>52%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

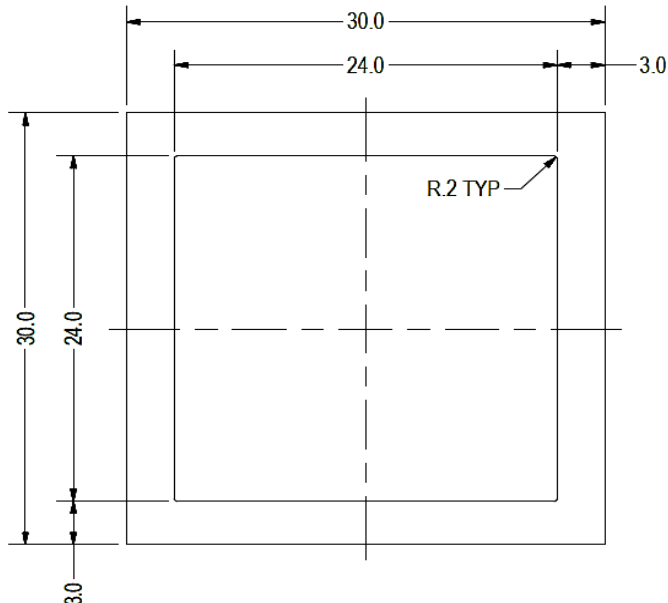
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.59 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.613 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.282 k
$M_{y \text{ allowable}}$ =	1.452 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>44%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.035 k-ft
$P_n$ =	0.175 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>10%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.778 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>20%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.678 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<b>15%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.004 k-ft
$P_n$ =	0.203 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>10%</b>



A cross brace kit is required every 19 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

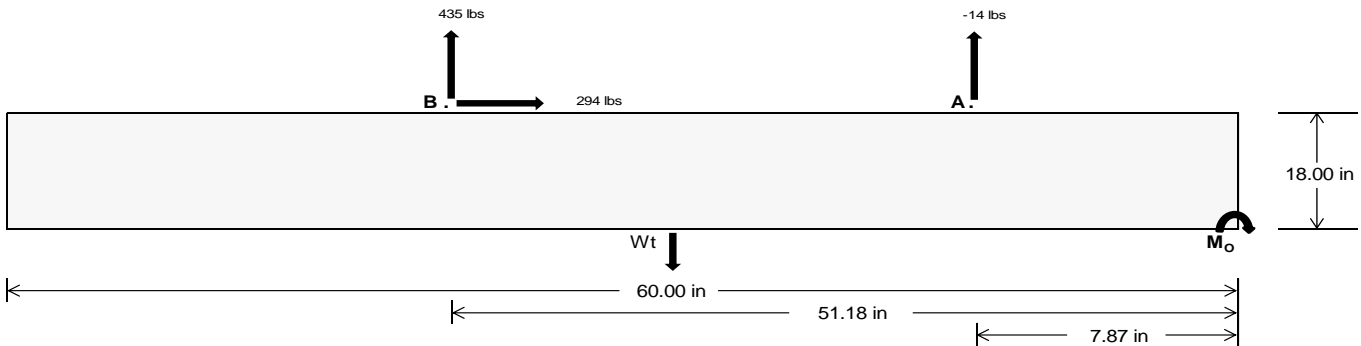
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =		<b>9.32</b>	<b>1886.98</b> k
Compressive Load =		<b>1027.54</b>	<b>1292.84</b> k
Lateral Load =		<b>28.99</b>	<b>1273.65</b> k
Moment (Weak Axis) =		<b>0.05</b>	<b>0.00</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 27418.9$  in-lbs  
Resisting Force Required = 913.96 lbs  
S.F. = 1.67  
Weight Required = 1523.27 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 293.75 lbs  
Friction = 0.4  
Weight Required = 734.38 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 293.75 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	378 lbs	378 lbs	378 lbs	378 lbs	337 lbs	337 lbs	337 lbs	337 lbs	498 lbs	498 lbs	498 lbs	498 lbs	28 lbs	28 lbs	28 lbs	28 lbs
$F_B$	250 lbs	250 lbs	250 lbs	250 lbs	552 lbs	552 lbs	552 lbs	552 lbs	575 lbs	575 lbs	575 lbs	575 lbs	-869 lbs	-869 lbs	-869 lbs	-869 lbs
$F_V$	44 lbs	44 lbs	44 lbs	44 lbs	533 lbs	533 lbs	533 lbs	533 lbs	429 lbs	429 lbs	429 lbs	429 lbs	-588 lbs	-588 lbs	-588 lbs	-588 lbs
$P_{total}$	2531 lbs	2621 lbs	2712 lbs	2802 lbs	2792 lbs	2883 lbs	2973 lbs	3064 lbs	2976 lbs	3066 lbs	3157 lbs	3248 lbs	301 lbs	355 lbs	410 lbs	464 lbs
$M$	322 lbs-ft	322 lbs-ft	322 lbs-ft	322 lbs-ft	447 lbs-ft	447 lbs-ft	447 lbs-ft	447 lbs-ft	547 lbs-ft	547 lbs-ft	547 lbs-ft	547 lbs-ft	705 lbs-ft	705 lbs-ft	705 lbs-ft	705 lbs-ft
$e$	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.34 ft	1.98 ft	1.72 ft	1.52 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	245.0 psf	243.8 psf	242.6 psf	241.6 psf	257.7 psf	255.9 psf	254.2 psf	252.7 psf	265.1 psf	263.0 psf	261.0 psf	259.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	333.4 psf	328.1 psf	323.3 psf	318.9 psf	380.4 psf	373.0 psf	366.3 psf	360.1 psf	415.1 psf	406.1 psf	397.9 psf	390.4 psf	727.8 psf	250.3 psf	182.8 psf	157.7 psf

Maximum Bearing Pressure = 728 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

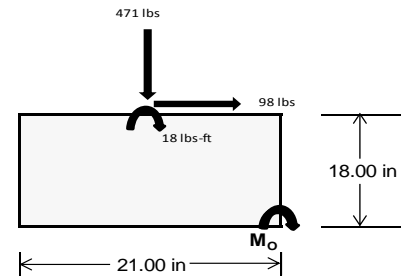
### Overturning Check

$M_o = 246.4$  ft-lbs  
 Resisting Force Required = 281.64 lbs  
 S.F. = 1.67  
 Weight Required = 469.40 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	133 lbs	87 lbs	69 lbs	243 lbs	471 lbs	194 lbs	86 lbs	-26 lbs	24 lbs
$F_v$	16 lbs	131 lbs	16 lbs	11 lbs	98 lbs	13 lbs	16 lbs	130 lbs	16 lbs
$P_{total}$	2489 lbs	2443 lbs	2425 lbs	2486 lbs	2714 lbs	2437 lbs	775 lbs	663 lbs	713 lbs
$M$	46 lbs-ft	220 lbs-ft	48 lbs-ft	32 lbs-ft	166 lbs-ft	37 lbs-ft	47 lbs-ft	220 lbs-ft	47 lbs-ft
$e$	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.06 ft	0.33 ft	0.07 ft
$L/6$	0.29 ft	1.57 ft	1.71 ft	1.72 ft	1.63 ft	1.72 ft	1.63 ft	1.09 ft	1.62 ft
$f_{min}$	266.4 sqft	193.1 sqft	258.5 sqft	271.7 sqft	245.2 sqft	264.0 sqft	70.4 sqft	-10.3 sqft	63.0 sqft
$f_{max}$	302.5 psf	365.3 psf	295.7 psf	296.5 psf	375.1 psf	293.0 psf	106.8 psf	161.8 psf	100.1 psf



Maximum Bearing Pressure = 375 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.597 k
Allowable Uplift =	1.214 k
Utilization =	<u>49%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.118 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.790 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>

#### Diagonal Strut

Maximum Axial Load =	0.778 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



#### Rear Strut

Maximum Axial Load =	1.164 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.203 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	33.11 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.662 in
Max Drift, $\Delta_{MAX}$ =	0.074 in
	<u>0.074 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$164.048$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$170.354$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.2 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.243 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.20 \\ &22.3976 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.20 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.452 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	214.551	1	-0.031	15	.24	1	0	10	0	4	0	15
30			min	-368.898	3	-.128	4	-.525	5	0	4	0	3	0	6
31		16	max	214.686	1	-.044	15	.24	1	0	10	0	4	0	15
32			min	-368.796	3	-.186	4	-.648	5	0	4	0	3	0	6
33		17	max	214.821	1	-.058	15	.24	1	0	10	0	4	0	15
34			min	-368.695	3	-.243	4	-.771	5	0	4	0	3	0	6
35		18	max	214.956	1	-.071	15	.24	1	0	10	0	1	0	15
36			min	-368.594	3	-.301	4	-.894	5	0	4	0	3	0	6
37		19	max	215.091	1	-.085	15	.24	1	0	10	0	1	0	15
38			min	-368.493	3	-.358	4	-1.018	5	0	4	0	3	0	6
39	M3	1	max	220.48	2	1.734	6	-.023	10	0	5	0	1	0	6
40			min	-219.164	3	.407	15	-1.35	4	0	1	0	10	0	15
41		2	max	220.41	2	1.558	6	-.023	10	0	5	0	1	0	2
42			min	-219.217	3	.366	15	-1.216	4	0	1	0	10	0	3
43		3	max	220.34	2	1.381	6	-.023	10	0	5	0	1	0	2
44			min	-219.269	3	.324	15	-1.082	4	0	1	0	5	0	3
45		4	max	220.27	2	1.205	6	-.023	10	0	5	0	1	0	15
46			min	-219.322	3	.283	15	-.949	4	0	1	0	5	0	4
47		5	max	220.2	2	1.028	6	-.023	10	0	5	0	1	0	15
48			min	-219.374	3	.241	15	-.815	4	0	1	0	5	0	4
49		6	max	220.13	2	.852	6	-.023	10	0	5	0	1	0	15
50			min	-219.427	3	.2	15	-.681	4	0	1	0	5	0	4
51		7	max	220.06	2	.676	6	-.023	10	0	5	0	1	0	15
52			min	-219.479	3	.158	15	-.548	4	0	1	0	5	0	4
53		8	max	219.99	2	.499	6	-.023	10	0	5	0	1	0	15
54			min	-219.532	3	.117	15	-.414	4	0	1	0	5	-.001	4
55		9	max	219.92	2	.323	6	-.023	10	0	5	0	1	0	15
56			min	-219.584	3	.075	15	-.28	4	0	1	0	5	-.001	4
57		10	max	219.85	2	.147	6	-.023	10	0	5	0	1	0	15
58			min	-219.637	3	.034	15	-.27	1	0	1	0	5	-.001	4
59		11	max	219.78	2	.005	2	.051	5	0	5	0	1	0	15
60			min	-219.689	3	-.054	3	-.27	1	0	1	0	5	-.001	4
61		12	max	219.71	2	-.049	15	.185	5	0	5	0	1	0	15
62			min	-219.742	3	-.206	4	-.27	1	0	1	0	5	-.001	4
63		13	max	219.64	2	-.09	15	.318	5	0	5	0	1	0	15
64			min	-219.794	3	-.383	4	-.27	1	0	1	0	5	-.001	4
65		14	max	219.57	2	-.132	15	.452	5	0	5	0	1	0	15
66			min	-219.847	3	-.559	4	-.27	1	0	1	0	5	-.001	4
67		15	max	219.5	2	-.173	15	.586	5	0	5	0	1	0	15
68			min	-219.899	3	-.735	4	-.27	1	0	1	0	5	0	4
69		16	max	219.43	2	-.215	15	.719	5	0	5	0	1	0	15
70			min	-219.952	3	-.912	4	-.27	1	0	1	0	5	0	4
71		17	max	219.36	2	-.256	15	.853	5	0	5	0	10	0	15
72			min	-220.004	3	-1.088	4	-.27	1	0	1	0	5	0	4
73		18	max	219.29	2	-.298	15	.987	5	0	5	0	10	0	15
74			min	-220.057	3	-1.264	4	-.27	1	0	1	0	4	0	4
75		19	max	219.22	2	-.339	15	1.12	5	0	5	0	5	0	1
76			min	-220.109	3	-1.441	4	-.27	1	0	1	0	1	0	1
77	M4	1	max	304.895	1	0	1	-.1	10	0	1	0	5	0	1
78			min	22.466	15	0	1	-21.283	4	0	1	0	2	0	1
79		2	max	304.959	1	0	1	-.1	10	0	1	0	12	0	1
80			min	22.486	15	0	1	-21.339	4	0	1	-.002	4	0	1
81		3	max	305.024	1	0	1	-.1	10	0	1	0	10	0	1
82			min	22.505	15	0	1	-21.395	4	0	1	-.004	4	0	1
83		4	max	305.089	1	0	1	-.1	10	0	1	0	10	0	1
84			min	22.525	15	0	1	-21.451	4	0	1	-.006	4	0	1
85		5	max	305.153	1	0	1	-.1	10	0	1	0	10	0	1

***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	22.544	15	0	1	-21.507	4	0	1	-.008	4	0	1
87		6	max	305.218	1	0	1	-.1	10	0	1	0	10	0	1
88			min	22.564	15	0	1	-21.563	4	0	1	-.01	4	0	1
89		7	max	305.283	1	0	1	-.1	10	0	1	0	10	0	1
90			min	22.583	15	0	1	-21.619	4	0	1	-.011	4	0	1
91		8	max	305.348	1	0	1	-.1	10	0	1	0	10	0	1
92			min	22.603	15	0	1	-21.675	4	0	1	-.013	4	0	1
93		9	max	305.412	1	0	1	-.1	10	0	1	0	10	0	1
94			min	22.622	15	0	1	-21.731	4	0	1	-.015	4	0	1
95		10	max	305.477	1	0	1	-.1	10	0	1	0	10	0	1
96			min	22.642	15	0	1	-21.787	4	0	1	-.017	4	0	1
97		11	max	305.542	1	0	1	-.1	10	0	1	0	10	0	1
98			min	22.661	15	0	1	-21.843	4	0	1	-.019	4	0	1
99		12	max	305.606	1	0	1	-.1	10	0	1	0	10	0	1
100			min	22.681	15	0	1	-21.899	4	0	1	-.021	4	0	1
101		13	max	305.671	1	0	1	-.1	10	0	1	0	10	0	1
102			min	22.701	15	0	1	-21.956	4	0	1	-.023	4	0	1
103		14	max	305.736	1	0	1	-.1	10	0	1	0	10	0	1
104			min	22.72	15	0	1	-22.012	4	0	1	-.025	4	0	1
105		15	max	305.801	1	0	1	-.1	10	0	1	0	10	0	1
106			min	22.74	15	0	1	-22.068	4	0	1	-.027	4	0	1
107		16	max	305.865	1	0	1	-.1	10	0	1	0	10	0	1
108			min	22.759	15	0	1	-22.124	4	0	1	-.029	4	0	1
109		17	max	305.93	1	0	1	-.1	10	0	1	0	10	0	1
110			min	22.779	15	0	1	-22.18	4	0	1	-.031	4	0	1
111		18	max	305.995	1	0	1	-.1	10	0	1	0	10	0	1
112			min	22.798	15	0	1	-22.236	4	0	1	-.033	4	0	1
113		19	max	306.059	1	0	1	-.1	10	0	1	0	10	0	1
114			min	22.818	15	0	1	-22.292	4	0	1	-.035	4	0	1
115	M6	1	max	675.227	1	.662	6	1.17	4	0	3	0	3	0	1
116			min	-1163.949	3	.147	15	-.215	3	0	5	0	9	0	1
117		2	max	675.362	1	.604	6	1.046	4	0	3	0	4	0	15
118			min	-1163.847	3	.133	15	-.215	3	0	5	0	9	0	6
119		3	max	675.497	1	.547	6	.923	4	0	3	0	4	0	15
120			min	-1163.746	3	.12	15	-.215	3	0	5	0	9	0	6
121		4	max	675.631	1	.491	2	.8	4	0	3	0	4	0	15
122			min	-1163.645	3	.106	15	-.215	3	0	5	0	10	0	6
123		5	max	675.766	1	.447	2	.677	4	0	3	0	4	0	15
124			min	-1163.544	3	.093	15	-.215	3	0	5	0	2	0	6
125		6	max	675.901	1	.402	2	.554	4	0	3	0	4	0	15
126			min	-1163.443	3	.076	12	-.215	3	0	5	0	2	0	6
127		7	max	676.036	1	.357	2	.431	4	0	3	.001	4	0	15
128			min	-1163.342	3	.054	12	-.215	3	0	5	0	2	0	2
129		8	max	676.171	1	.312	2	.307	4	0	3	.001	4	0	15
130			min	-1163.241	3	.032	12	-.215	3	0	5	0	3	0	2
131		9	max	676.306	1	.267	2	.184	4	0	3	.001	4	0	15
132			min	-1163.139	3	.001	3	-.215	3	0	5	0	3	0	2
133		10	max	676.441	1	.223	2	.062	14	0	3	.001	4	0	15
134			min	-1163.038	3	-.032	3	-.215	3	0	5	0	3	0	2
135		11	max	676.576	1	.178	2	.06	9	0	3	.001	4	0	12
136			min	-1162.937	3	-.066	3	-.215	3	0	5	0	3	0	2
137		12	max	676.71	1	.133	2	.06	9	0	3	.001	4	0	12
138			min	-1162.836	3	-.099	3	-.215	3	0	5	0	3	0	2
139		13	max	676.845	1	.088	2	.06	9	0	3	.001	4	0	12
140			min	-1162.735	3	-.133	3	-.329	5	0	5	0	3	0	2
141		14	max	676.98	1	.043	2	.06	9	0	3	.001	4	0	12
142			min	-1162.634	3	-.167	3	-.452	5	0	5	0	3	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	677.115	1	-0.001	2	.06	9	0	3	0	4	0	12
144		min	-1162.532	3	-.2	3	-.575	5	0	5	0	3	0	2
145	16	max	677.25	1	-.046	2	.06	9	0	3	0	4	0	3
146		min	-1162.431	3	-.234	3	-.698	5	0	5	0	3	0	2
147	17	max	677.385	1	-.07	15	.06	9	0	3	0	4	0	3
148		min	-1162.33	3	-.267	3	-.821	5	0	5	0	3	0	2
149	18	max	677.52	1	-.083	15	.06	9	0	3	0	4	0	3
150		min	-1162.229	3	-.316	4	-.945	5	0	5	0	3	0	2
151	19	max	677.654	1	-.097	15	.06	9	0	3	0	4	0	3
152		min	-1162.128	3	-.374	4	-1.068	5	0	5	0	3	0	2
153	M7	1	max	778.215	2	1.757	.041	3	0	1	0	4	0	2
154		min	-668.462	3	.42	15	-1.297	4	0	3	0	3	0	3
155	2	max	778.145	2	1.58	4	.041	3	0	1	0	4	0	2
156		min	-668.514	3	.379	15	-1.163	4	0	3	0	3	0	3
157	3	max	778.075	2	1.404	4	.041	3	0	1	0	2	0	2
158		min	-668.567	3	.338	15	-1.03	4	0	3	0	3	0	3
159	4	max	778.005	2	1.227	4	.041	3	0	1	0	2	0	2
160		min	-668.619	3	.296	15	-.896	4	0	3	0	3	0	3
161	5	max	777.935	2	1.051	4	.041	3	0	1	0	1	0	15
162		min	-668.672	3	.255	15	-.762	4	0	3	0	5	0	3
163	6	max	777.865	2	.875	4	.041	3	0	1	0	1	0	15
164		min	-668.724	3	.213	15	-.629	4	0	3	0	5	0	3
165	7	max	777.795	2	.698	4	.041	3	0	1	0	1	0	15
166		min	-668.777	3	.172	15	-.495	4	0	3	0	5	0	6
167	8	max	777.725	2	.522	4	.041	3	0	1	0	1	0	15
168		min	-668.829	3	.125	12	-.361	4	0	3	0	5	-.001	6
169	9	max	777.655	2	.346	2	.041	3	0	1	0	1	0	15
170		min	-668.882	3	.057	12	-.228	4	0	3	0	5	-.001	6
171	10	max	777.585	2	.208	2	.041	3	0	1	0	1	0	15
172		min	-668.934	3	-.03	3	-.094	4	0	3	0	5	-.001	6
173	11	max	777.515	2	.071	2	.041	3	0	1	0	1	0	15
174		min	-668.987	3	-.133	3	-.017	2	0	3	0	5	-.001	6
175	12	max	777.445	2	-.036	15	.173	5	0	1	0	1	0	15
176		min	-669.039	3	-.236	3	-.017	2	0	3	0	5	-.001	6
177	13	max	777.375	2	-.077	15	.307	5	0	1	0	1	0	15
178		min	-669.092	3	-.361	6	-.017	2	0	3	0	5	-.001	6
179	14	max	777.305	2	-.119	15	.441	5	0	1	0	1	0	15
180		min	-669.144	3	-.537	6	-.017	2	0	3	0	5	-.001	6
181	15	max	777.235	2	-.16	15	.574	5	0	1	0	1	0	15
182		min	-669.197	3	-.713	6	-.017	2	0	3	0	5	0	6
183	16	max	777.165	2	-.201	15	.708	5	0	1	0	1	0	15
184		min	-669.249	3	-.89	6	-.017	2	0	3	0	5	0	6
185	17	max	777.095	2	-.243	15	.842	5	0	1	0	1	0	15
186		min	-669.302	3	-1.066	6	-.017	2	0	3	0	5	0	6
187	18	max	777.025	2	-.284	15	.975	5	0	1	0	1	0	15
188		min	-669.354	3	-1.242	6	-.017	2	0	3	0	5	0	6
189	19	max	776.955	2	-.326	15	1.109	5	0	1	0	1	0	1
190		min	-669.407	3	-1.419	6	-.017	2	0	3	0	3	0	1
191	M8	1	max	789.251	1	0	.476	1	0	1	0	4	0	1
192		min	25.403	15	0	1	-21.468	4	0	1	0	3	0	1
193	2	max	789.316	1	0	1	.476	1	0	1	0	1	0	1
194		min	25.423	15	0	1	-21.524	4	0	1	-.002	4	0	1
195	3	max	789.38	1	0	1	.476	1	0	1	0	1	0	1
196		min	25.442	15	0	1	-21.58	4	0	1	-.004	4	0	1
197	4	max	789.445	1	0	1	.476	1	0	1	0	1	0	1
198		min	25.462	15	0	1	-21.636	4	0	1	-.006	4	0	1
199	5	max	789.51	1	0	1	.476	1	0	1	0	1	0	1

***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	25.481	15	0	1	-21.692	4	0	1	-.008	4	0	1
201		6	max	789.575	1	0	1	.476	1	0	1	0	1	0	1
202			min	25.501	15	0	1	-21.748	4	0	1	-.01	4	0	1
203		7	max	789.639	1	0	1	.476	1	0	1	0	1	0	1
204			min	25.52	15	0	1	-21.804	4	0	1	-.012	4	0	1
205		8	max	789.704	1	0	1	.476	1	0	1	0	1	0	1
206			min	25.54	15	0	1	-21.861	4	0	1	-.014	4	0	1
207		9	max	789.769	1	0	1	.476	1	0	1	0	1	0	1
208			min	25.559	15	0	1	-21.917	4	0	1	-.015	4	0	1
209		10	max	789.833	1	0	1	.476	1	0	1	0	1	0	1
210			min	25.579	15	0	1	-21.973	4	0	1	-.017	4	0	1
211		11	max	789.898	1	0	1	.476	1	0	1	0	1	0	1
212			min	25.598	15	0	1	-22.029	4	0	1	-.019	4	0	1
213		12	max	789.963	1	0	1	.476	1	0	1	0	1	0	1
214			min	25.618	15	0	1	-22.085	4	0	1	-.021	4	0	1
215		13	max	790.027	1	0	1	.476	1	0	1	0	1	0	1
216			min	25.638	15	0	1	-22.141	4	0	1	-.023	4	0	1
217		14	max	790.092	1	0	1	.476	1	0	1	0	1	0	1
218			min	25.657	15	0	1	-22.197	4	0	1	-.025	4	0	1
219		15	max	790.157	1	0	1	.476	1	0	1	0	1	0	1
220			min	25.677	15	0	1	-22.253	4	0	1	-.027	4	0	1
221		16	max	790.222	1	0	1	.476	1	0	1	0	1	0	1
222			min	25.696	15	0	1	-22.309	4	0	1	-.029	4	0	1
223		17	max	790.286	1	0	1	.476	1	0	1	0	1	0	1
224			min	25.716	15	0	1	-22.365	4	0	1	-.031	4	0	1
225		18	max	790.351	1	0	1	.476	1	0	1	0	1	0	1
226			min	25.735	15	0	1	-22.421	4	0	1	-.033	4	0	1
227		19	max	790.416	1	0	1	.476	1	0	1	0	1	0	1
228			min	25.755	15	0	1	-22.477	4	0	1	-.035	4	0	1
229	M10	1	max	215.058	1	.709	4	1.307	5	0	1	0	1	0	1
230			min	-318.376	3	.18	15	-.139	1	-.001	5	0	3	0	1
231		2	max	215.193	1	.652	4	1.184	5	0	1	0	1	0	15
232			min	-318.274	3	.167	15	-.139	1	-.001	5	0	3	0	4
233		3	max	215.328	1	.594	4	1.061	5	0	1	0	4	0	15
234			min	-318.173	3	.153	15	-.139	1	-.001	5	0	3	0	4
235		4	max	215.463	1	.537	4	.938	5	0	1	0	4	0	15
236			min	-318.072	3	.14	15	-.139	1	-.001	5	0	3	0	4
237		5	max	215.598	1	.479	4	.815	5	0	1	0	4	0	15
238			min	-317.971	3	.126	15	-.139	1	-.001	5	0	3	0	4
239		6	max	215.733	1	.422	4	.692	5	0	1	0	4	0	15
240			min	-317.87	3	.113	15	-.139	1	-.001	5	0	3	0	4
241		7	max	215.867	1	.364	4	.568	5	0	1	0	4	0	15
242			min	-317.769	3	.099	15	-.139	1	-.001	5	0	3	0	4
243		8	max	216.002	1	.307	4	.445	5	0	1	.001	4	0	15
244			min	-317.668	3	.086	15	-.139	1	-.001	5	0	3	0	4
245		9	max	216.137	1	.249	4	.322	5	0	1	.001	4	0	15
246			min	-317.566	3	.072	15	-.139	1	-.001	5	0	3	0	4
247		10	max	216.272	1	.192	4	.199	5	0	1	.001	4	0	15
248			min	-317.465	3	.05	12	-.139	1	-.001	5	0	3	0	4
249		11	max	216.407	1	.134	4	.076	5	0	1	.001	4	0	15
250			min	-317.364	3	.028	12	-.139	1	-.001	5	0	3	0	4
251		12	max	216.542	1	.077	4	.007	3	0	1	.001	4	0	15
252			min	-317.263	3	.006	12	-.139	1	-.001	5	0	3	0	4
253		13	max	216.677	1	.027	5	.007	3	0	1	.001	5	0	15
254			min	-317.162	3	-.027	3	-.194	4	-.001	5	0	3	0	4
255		14	max	216.811	1	.006	5	.007	3	0	1	.001	5	0	15
256			min	-317.061	3	-.061	3	-.317	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	216.946	1	-0.009	15	.007	3	0	1	.001	5	0	15
258		min	-316.96	3	-.097	6	-.44	4	-.001	5	0	3	0	4
259	16	max	217.081	1	-.022	15	.007	3	0	1	0	5	0	15
260		min	-316.858	3	-.154	6	-.563	4	-.001	5	0	3	0	4
261	17	max	217.216	1	-.036	15	.007	3	0	1	0	5	0	15
262		min	-316.757	3	-.212	6	-.686	4	-.001	5	0	3	0	4
263	18	max	217.351	1	-.049	15	.007	3	0	1	0	5	0	12
264		min	-316.656	3	-.269	6	-.809	4	-.001	5	0	1	0	4
265	19	max	217.486	1	-.063	15	.007	3	0	1	0	5	0	12
266		min	-316.555	3	-.327	6	-.933	4	-.001	5	0	1	0	4
267	M11	1	max	219.995	2	1.721	.289	1	.001	4	0	5	0	2
268		min	-219.887	3	.398	15	-1.245	5	0	10	0	1	0	15
269	2	max	219.925	2	1.544	6	.289	1	.001	4	0	5	0	2
270		min	-219.94	3	.356	15	-1.112	5	0	10	0	1	0	3
271	3	max	219.855	2	1.368	6	.289	1	.001	4	0	3	0	2
272		min	-219.992	3	.315	15	-.978	5	0	10	0	1	0	3
273	4	max	219.785	2	1.192	6	.289	1	.001	4	0	3	0	15
274		min	-220.045	3	.273	15	-.844	5	0	10	0	1	0	4
275	5	max	219.715	2	1.015	6	.289	1	.001	4	0	3	0	15
276		min	-220.097	3	.232	15	-.711	5	0	10	0	1	0	4
277	6	max	219.645	2	.839	6	.289	1	.001	4	0	3	0	15
278		min	-220.15	3	.19	15	-.577	5	0	10	0	1	0	4
279	7	max	219.575	2	.663	6	.289	1	.001	4	0	3	0	15
280		min	-220.202	3	.149	15	-.443	5	0	10	0	1	-.001	4
281	8	max	219.505	2	.486	6	.289	1	.001	4	0	3	0	15
282		min	-220.255	3	.108	15	-.31	5	0	10	0	4	-.001	4
283	9	max	219.435	2	.31	6	.289	1	.001	4	0	3	0	15
284		min	-220.307	3	.066	15	-.176	5	0	10	0	4	-.001	4
285	10	max	219.365	2	.142	2	.289	1	.001	4	0	3	0	15
286		min	-220.36	3	.025	15	-.049	3	0	10	0	4	-.001	4
287	11	max	219.295	2	.005	2	.289	1	.001	4	0	3	0	15
288		min	-220.412	3	-.067	3	-.049	3	0	10	0	4	-.001	4
289	12	max	219.225	2	-.058	15	.293	4	.001	4	0	3	0	15
290		min	-220.465	3	-.22	4	-.049	3	0	10	0	4	-.001	4
291	13	max	219.155	2	-.1	15	.427	4	.001	4	0	3	0	15
292		min	-220.517	3	-.396	4	-.049	3	0	10	0	4	-.001	4
293	14	max	219.085	2	-.141	15	.561	4	.001	4	0	3	0	15
294		min	-220.57	3	-.573	4	-.049	3	0	10	0	4	-.001	4
295	15	max	219.015	2	-.183	15	.694	4	.001	4	0	3	0	15
296		min	-220.622	3	-.749	4	-.049	3	0	10	0	4	0	4
297	16	max	218.945	2	-.224	15	.828	4	.001	4	0	3	0	15
298		min	-220.675	3	-.925	4	-.049	3	0	10	0	5	0	4
299	17	max	218.875	2	-.266	15	.962	4	.001	4	0	3	0	15
300		min	-220.727	3	-1.102	4	-.049	3	0	10	0	10	0	4
301	18	max	218.805	2	-.307	15	1.095	4	.001	4	0	4	0	15
302		min	-220.78	3	-1.278	4	-.049	3	0	10	0	10	0	4
303	19	max	218.735	2	-.349	15	1.229	4	.001	4	0	4	0	1
304		min	-220.832	3	-1.455	4	-.049	3	0	10	0	10	0	1
305	M12	1	max	304.771	1	0	1.869	1	0	1	0	4	0	1
306		min	3.958	15	0	1	-19.68	5	0	1	0	3	0	1
307	2	max	304.836	1	0	1	1.869	1	0	1	0	1	0	1
308		min	3.978	15	0	1	-19.736	5	0	1	-.002	5	0	1
309	3	max	304.9	1	0	1	1.869	1	0	1	0	1	0	1
310		min	3.997	15	0	1	-19.792	5	0	1	-.003	5	0	1
311	4	max	304.965	1	0	1	1.869	1	0	1	0	1	0	1
312		min	4.017	15	0	1	-19.848	5	0	1	-.005	5	0	1
313	5	max	305.03	1	0	1	1.869	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	4.036	15	0	1	-19.904	5	0	1	-.007	5	0	1
315	6	max	305.095	1	0	1	1.869	1	0	1	0	1	0	1
316		min	4.056	15	0	1	-19.96	5	0	1	-.009	5	0	1
317	7	max	305.159	1	0	1	1.869	1	0	1	.001	1	0	1
318		min	4.075	15	0	1	-20.016	5	0	1	-.011	5	0	1
319	8	max	305.224	1	0	1	1.869	1	0	1	.001	1	0	1
320		min	4.095	15	0	1	-20.072	5	0	1	-.012	5	0	1
321	9	max	305.289	1	0	1	1.869	1	0	1	.001	1	0	1
322		min	4.114	15	0	1	-20.128	5	0	1	-.014	5	0	1
323	10	max	305.353	1	0	1	1.869	1	0	1	.002	1	0	1
324		min	4.134	15	0	1	-20.184	5	0	1	-.016	5	0	1
325	11	max	305.418	1	0	1	1.869	1	0	1	.002	1	0	1
326		min	4.153	15	0	1	-20.241	5	0	1	-.018	5	0	1
327	12	max	305.483	1	0	1	1.869	1	0	1	.002	1	0	1
328		min	4.173	15	0	1	-20.297	5	0	1	-.02	5	0	1
329	13	max	305.548	1	0	1	1.869	1	0	1	.002	1	0	1
330		min	4.192	15	0	1	-20.353	5	0	1	-.021	5	0	1
331	14	max	305.612	1	0	1	1.869	1	0	1	.002	1	0	1
332		min	4.212	15	0	1	-20.409	5	0	1	-.023	5	0	1
333	15	max	305.677	1	0	1	1.869	1	0	1	.002	1	0	1
334		min	4.231	15	0	1	-20.465	5	0	1	-.025	5	0	1
335	16	max	305.742	1	0	1	1.869	1	0	1	.003	1	0	1
336		min	4.251	15	0	1	-20.521	5	0	1	-.027	5	0	1
337	17	max	305.806	1	0	1	1.869	1	0	1	.003	1	0	1
338		min	4.27	15	0	1	-20.577	5	0	1	-.029	5	0	1
339	18	max	305.871	1	0	1	1.869	1	0	1	.003	1	0	1
340		min	4.29	15	0	1	-20.633	5	0	1	-.031	5	0	1
341	19	max	305.936	1	0	1	1.869	1	0	1	.003	1	0	1
342		min	4.309	15	0	1	-20.689	5	0	1	-.032	5	0	1
343	M1	1	max	106.242	1	346.194	3	-3.01	10	0	.074	1	0	2
344		min	7.055	12	-229.612	2	-37.758	1	0	3	.006	10	0	3
345	2	max	106.402	1	346.023	3	-3.01	10	0	2	.066	1	.05	2
346		min	7.135	12	-229.841	2	-37.758	1	0	3	.005	10	-.075	3
347	3	max	117.421	3	5.387	14	-2.994	10	0	12	.057	1	.099	2
348		min	-18.617	10	-29.625	2	-37.666	1	0	1	.005	10	-.149	3
349	4	max	117.541	3	5.162	14	-2.994	10	0	12	.049	1	.106	2
350		min	-18.484	10	-29.854	2	-37.666	1	0	1	.004	10	-.147	3
351	5	max	117.661	3	4.937	14	-2.994	10	0	12	.041	1	.112	2
352		min	-18.35	10	-30.083	2	-37.666	1	0	1	.003	10	-.145	3
353	6	max	117.781	3	4.712	14	-2.994	10	0	12	.033	1	.119	2
354		min	-18.217	10	-30.311	2	-37.666	1	0	1	.003	10	-.143	3
355	7	max	117.901	3	4.488	14	-2.994	10	0	12	.025	1	.125	2
356		min	-18.083	10	-30.54	2	-37.666	1	0	1	.002	10	-.141	3
357	8	max	118.021	3	4.29	9	-2.994	10	0	12	.016	1	.132	2
358		min	-17.95	10	-30.769	2	-37.666	1	0	1	.001	10	-.139	3
359	9	max	118.141	3	4.099	9	-2.994	10	0	12	.008	1	.139	2
360		min	-17.816	10	-30.997	2	-37.666	1	0	1	0	10	-.137	3
361	10	max	118.261	3	3.909	9	-2.994	10	0	12	.002	3	.145	2
362		min	-17.683	10	-31.226	2	-37.666	1	0	1	0	10	-.134	3
363	11	max	118.382	3	3.718	9	-2.994	10	0	12	0	3	.152	2
364		min	-17.549	10	-31.455	2	-37.666	1	0	1	-.008	1	-.132	3
365	12	max	118.502	3	3.528	9	-2.994	10	0	12	0	12	.159	2
366		min	-17.416	10	-31.684	2	-37.666	1	0	1	-.016	1	-.13	3
367	13	max	118.622	3	3.337	9	-2.994	10	0	12	-.001	12	.166	2
368		min	-17.282	10	-31.912	2	-37.666	1	0	1	-.024	1	-.128	3
369	14	max	118.742	3	3.146	9	-2.994	10	0	12	-.002	12	.173	2
370		min	-17.149	10	-32.141	2	-37.666	1	0	1	-.033	1	-.125	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	118.862	3	2.956	9	-2.994	10	0	12	-.003	12	.18	2
372			min	-17.015	10	-32.37	2	-37.666	1	0	1	-.041	1	-.123	3
373		16	max	89.06	2	160.783	2	-3.014	10	0	1	-.003	12	.185	2
374			min	2.576	15	-205.948	3	-37.893	1	0	5	-.049	1	-.119	3
375		17	max	89.22	2	160.554	2	-3.014	10	0	1	-.004	12	.15	2
376			min	2.624	15	-206.12	3	-37.893	1	0	5	-.057	1	-.074	3
377		18	max	-5.565	12	347.542	2	-3.115	10	0	3	-.005	12	.076	2
378			min	-106.392	1	-170.367	3	-38.925	4	0	2	-.066	1	-.037	3
379		19	max	-5.485	12	347.313	2	-3.115	10	0	3	-.006	12	0	2
380			min	-106.232	1	-170.539	3	-38.858	1	0	2	-.074	1	0	3
381	M5	1	max	243.958	1	1129.822	3	0	2	0	1	.035	4	0	3
382			min	6.228	12	-745.183	2	-68.269	3	0	5	0	2	0	2
383		2	max	244.118	1	1129.651	3	0	2	0	1	.03	4	.161	2
384			min	6.308	12	-745.412	2	-68.269	3	0	5	-.006	3	-.245	3
385		3	max	356.74	3	4.883	9	7.661	3	0	3	.025	4	.32	2
386			min	-84.892	2	-104.369	2	-19.061	4	0	4	-.02	3	-.484	3
387		4	max	356.861	3	4.693	9	7.661	3	0	3	.021	4	.343	2
388			min	-84.732	2	-104.597	2	-18.819	4	0	4	-.019	3	-.477	3
389		5	max	356.981	3	4.502	9	7.661	3	0	3	.017	4	.366	2
390			min	-84.572	2	-104.826	2	-18.577	4	0	4	-.017	3	-.469	3
391		6	max	357.101	3	4.311	9	7.661	3	0	3	.013	4	.388	2
392			min	-84.411	2	-105.055	2	-18.335	4	0	4	-.015	3	-.461	3
393		7	max	357.221	3	4.121	9	7.661	3	0	3	.009	4	.411	2
394			min	-84.251	2	-105.284	2	-18.093	4	0	4	-.014	3	-.453	3
395		8	max	357.341	3	3.93	9	7.661	3	0	3	.005	4	.434	2
396			min	-84.091	2	-105.512	2	-17.851	4	0	4	-.012	3	-.445	3
397		9	max	357.461	3	3.739	9	7.661	3	0	3	0	4	.457	2
398			min	-83.931	2	-105.741	2	-17.609	4	0	4	-.01	3	-.437	3
399		10	max	357.581	3	3.549	9	7.661	3	0	3	0	2	.48	2
400			min	-83.771	2	-105.97	2	-17.367	4	0	4	-.009	3	-.429	3
401		11	max	357.701	3	3.358	9	7.661	3	0	3	0	2	.503	2
402			min	-83.611	2	-106.199	2	-17.125	4	0	4	-.007	3	-.421	3
403		12	max	357.822	3	3.168	9	7.661	3	0	3	0	2	.526	2
404			min	-83.45	2	-106.427	2	-16.883	4	0	4	-.01	4	-.413	3
405		13	max	357.942	3	2.977	9	7.661	3	0	3	0	10	.549	2
406			min	-83.29	2	-106.656	2	-16.641	4	0	4	-.014	4	-.404	3
407		14	max	358.062	3	2.786	9	7.661	3	0	3	0	10	.572	2
408			min	-83.13	2	-106.885	2	-16.399	4	0	4	-.017	4	-.396	3
409		15	max	358.182	3	2.596	9	7.661	3	0	3	0	10	.596	2
410			min	-82.97	2	-107.114	2	-16.157	4	0	4	-.021	4	-.388	3
411		16	max	282.484	2	581.829	2	7.641	3	0	3	0	3	.613	2
412			min	1.799	15	-630.278	3	-14.815	4	0	4	-.025	4	-.374	3
413		17	max	282.644	2	581.601	2	7.641	3	0	3	.003	3	.487	2
414			min	1.847	15	-630.449	3	-14.573	4	0	4	-.028	4	-.238	3
415		18	max	-8.552	12	1128.189	2	6.982	3	0	4	.004	3	.244	2
416			min	-244.133	1	-548.148	3	-35.251	5	0	1	-.035	4	-.118	3
417		19	max	-8.472	12	1127.96	2	6.982	3	0	4	.006	3	0	3
418			min	-243.973	1	-548.319	3	-35.009	5	0	1	-.043	4	0	2
419	M9	1	max	105.891	1	346.126	3	149.702	4	0	3	-.001	15	0	2
420			min	2.874	15	-229.612	2	3.01	10	0	2	-.073	1	0	3
421		2	max	106.051	1	345.955	3	149.944	4	0	3	.028	5	.05	2
422			min	2.923	15	-229.841	2	3.01	10	0	2	-.065	1	-.075	3
423		3	max	117.243	3	5.224	9	36.844	1	0	1	.057	5	.099	2
424			min	-18.158	10	-29.629	2	-24.054	5	0	5	-.056	1	-.149	3
425		4	max	117.363	3	5.033	9	36.844	1	0	1	.052	5	.105	2
426			min	-18.024	10	-29.858	2	-23.812	5	0	5	-.048	1	-.147	3
427		5	max	117.483	3	4.843	9	36.844	1	0	1	.047	5	.112	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-17.891	10	-30.086	2	-23.57	5	0	5	-.04	1	-.145	3
429	6	max	117.603	3	4.652	9	36.844	1	0	1	.042	5	.119	2
430		min	-17.757	10	-30.315	2	-23.328	5	0	5	-.032	1	-.143	3
431	7	max	117.723	3	4.462	9	36.844	1	0	1	.037	5	.125	2
432		min	-17.624	10	-30.544	2	-23.086	5	0	5	-.024	1	-.141	3
433	8	max	117.844	3	4.271	9	36.844	1	0	1	.032	5	.132	2
434		min	-17.49	10	-30.772	2	-22.844	5	0	5	-.016	1	-.139	3
435	9	max	117.964	3	4.08	9	36.844	1	0	1	.027	5	.138	2
436		min	-17.357	10	-31.001	2	-22.602	5	0	5	-.008	1	-.137	3
437	10	max	118.084	3	3.89	9	36.844	1	0	1	.022	4	.145	2
438		min	-17.223	10	-31.23	2	-22.36	5	0	5	0	1	-.134	3
439	11	max	118.204	3	3.699	9	36.844	1	0	1	.019	4	.152	2
440		min	-17.09	10	-31.459	2	-22.118	5	0	5	0	10	-.132	3
441	12	max	118.324	3	3.509	9	36.844	1	0	1	.016	4	.159	2
442		min	-16.956	10	-31.687	2	-21.876	5	0	5	.001	10	-.13	3
443	13	max	118.444	3	3.318	9	36.844	1	0	1	.024	1	.166	2
444		min	-16.823	10	-31.916	2	-21.634	5	0	5	.002	10	-.128	3
445	14	max	118.564	3	3.127	9	36.844	1	0	1	.032	1	.173	2
446		min	-16.689	10	-32.145	2	-21.392	5	0	5	.002	15	-.126	3
447	15	max	118.684	3	2.937	9	36.844	1	0	1	.04	1	.18	2
448		min	-16.556	10	-32.374	2	-21.15	5	0	5	-.002	5	-.123	3
449	16	max	89.368	2	160.352	2	37.087	1	0	10	.048	1	.185	2
450		min	4.329	15	-206.494	3	-19.749	5	0	4	-.005	5	-.119	3
451	17	max	89.528	2	160.123	2	37.087	1	0	10	.056	1	.15	2
452		min	4.377	15	-206.666	3	-19.507	5	0	4	-.009	5	-.074	3
453	18	max	2.91	5	347.542	2	38.988	1	0	2	.065	1	.076	2
454		min	-106.048	1	-170.359	3	-38.914	5	0	3	-.018	5	-.037	3
455	19	max	2.985	5	347.313	2	38.988	1	0	2	.073	1	0	2
456		min	-105.888	1	-170.53	3	-38.672	5	0	3	-.026	5	0	3
457	M13	1	max	149.706	4	229.478	2	-2.874	15	0	.073	1	0	2
458		min	3.011	10	-346.156	3	-105.882	1	0	3	.001	15	0	3
459	2	max	143.977	4	162.649	2	-1.731	15	0	2	.019	1	.172	3
460		min	3.011	10	-244.936	3	-80.544	1	0	3	0	10	-.114	2
461	3	max	138.247	4	95.819	2	-.587	15	0	2	.008	3	.286	3
462		min	3.011	10	-143.717	3	-55.205	1	0	3	-.021	1	-.19	2
463	4	max	132.518	4	28.99	2	.748	5	0	2	.004	3	.34	3
464		min	3.011	10	-42.498	3	-29.867	1	0	3	-.046	1	-.226	2
465	5	max	126.789	4	58.722	3	2.518	5	0	2	.002	3	.335	3
466		min	3.011	10	-37.84	2	-4.528	1	0	3	-.056	1	-.224	2
467	6	max	121.059	4	159.941	3	20.81	1	0	2	.002	5	.272	3
468		min	3.011	10	-104.669	2	-2.063	3	0	3	-.051	1	-.182	2
469	7	max	115.33	4	261.16	3	46.149	1	0	2	.005	5	.149	3
470		min	3.011	10	-171.499	2	-.399	3	0	3	-.031	1	-.101	2
471	8	max	109.601	4	362.38	3	71.487	1	0	2	.009	4	.018	1
472		min	3.011	10	-238.329	2	1.068	12	0	3	0	3	-.033	3
473	9	max	103.872	4	463.599	3	96.826	1	0	2	.052	1	.177	2
474		min	3.011	10	-305.158	2	2.177	12	0	3	0	12	-.274	3
475	10	max	98.142	4	564.818	3	122.164	1	0	2	.116	1	.374	2
476		min	3.011	10	-345.962	1	-52.658	2	0	3	-.006	3	-.574	3
477	11	max	70.23	4	305.158	2	1.671	5	0	3	.051	1	.177	2
478		min	3.011	10	-463.599	3	-96.475	1	0	2	-.013	5	-.274	3
479	12	max	64.501	4	238.329	2	3.44	5	0	3	.004	2	.018	1
480		min	3.011	10	-362.379	3	-71.136	1	0	2	-.012	4	-.033	3
481	13	max	58.772	4	171.499	2	5.21	5	0	3	-.003	10	.149	3
482		min	3.011	10	-261.16	3	-45.798	1	0	2	-.032	1	-.101	2
483	14	max	53.043	4	104.669	2	6.979	5	0	3	-.004	15	.272	3
484		min	3.011	10	-159.941	3	-20.459	1	0	2	-.051	1	-.182	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	47.313	4	37.84	2	10.235	4	0	3	0	15	.335	3
486			min	3.011	10	-58.722	3	-.757	10	0	2	-.055	1	-.224	2
487		16	max	41.584	4	42.498	3	30.218	1	0	3	.004	5	.34	3
488			min	3.011	10	-28.99	2	2.403	10	0	2	-.045	1	-.226	2
489		17	max	37.853	1	143.717	3	55.556	1	0	3	.011	5	.286	3
490			min	3.011	10	-95.819	2	4.836	12	0	2	-.02	1	-.19	2
491		18	max	37.853	1	244.936	3	80.895	1	0	3	.024	4	.172	3
492			min	3.011	10	-162.649	2	5.946	12	0	2	0	10	-.114	2
493		19	max	37.853	1	346.156	3	106.233	1	0	3	.074	1	0	2
494			min	3.011	10	-229.478	2	7.056	12	0	2	.006	10	0	3
495	M16	1	max	38.666	5	347.486	2	2.985	5	0	3	.073	1	0	2
496			min	-38.889	1	-170.562	3	-105.897	1	0	2	-.026	5	0	3
497		2	max	32.937	5	246.124	2	4.754	5	0	3	.019	1	.085	3
498			min	-38.889	1	-121.141	3	-80.559	1	0	2	-.024	5	-.173	2
499		3	max	27.208	5	144.762	2	6.524	5	0	3	0	12	.141	3
500			min	-38.889	1	-71.72	3	-55.22	1	0	2	-.025	4	-.287	2
501		4	max	21.479	5	43.401	2	8.293	5	0	3	-.002	12	.169	3
502			min	-38.889	1	-22.299	3	-29.882	1	0	2	-.046	1	-.342	2
503		5	max	15.749	5	27.122	3	10.063	5	0	3	-.004	12	.167	3
504			min	-38.889	1	-57.961	2	-4.543	1	0	2	-.056	1	-.338	2
505		6	max	10.02	5	76.543	3	20.795	1	0	3	-.003	15	.137	3
506			min	-38.889	1	-159.323	2	-.925	3	0	2	-.051	1	-.274	2
507		7	max	4.291	5	125.964	3	46.134	1	0	3	.003	5	.078	3
508			min	-38.889	1	-260.685	2	.641	12	0	2	-.031	1	-.152	2
509		8	max	1.119	3	175.385	3	71.472	1	0	3	.012	4	.03	2
510			min	-38.889	1	-362.046	2	1.75	12	0	2	-.006	3	-.01	3
511		9	max	1.119	3	224.806	3	96.811	1	0	3	.052	1	.271	2
512			min	-38.889	1	-463.408	2	2.86	12	0	2	-.004	3	-.127	3
513		10	max	22.129	5	-9.098	15	122.149	1	0	14	.116	1	.57	2
514			min	-38.889	1	-564.77	2	-6.627	3	0	2	.003	12	-.272	3
515		11	max	16.4	5	463.408	2	1.249	5	0	2	.051	1	.271	2
516			min	-38.764	1	-224.806	3	-96.467	1	0	3	-.011	5	-.127	3
517		12	max	10.67	5	362.046	2	3.019	5	0	2	.004	2	.03	2
518			min	-38.764	1	-175.385	3	-71.128	1	0	3	-.01	5	-.01	3
519		13	max	4.941	5	260.685	2	4.788	5	0	2	-.002	12	.078	3
520			min	-38.764	1	-125.964	3	-45.79	1	0	3	-.032	1	-.152	2
521		14	max	-.46	15	159.323	2	6.558	5	0	2	-.002	12	.137	3
522			min	-38.764	1	-76.543	3	-20.451	1	0	3	-.051	1	-.274	2
523		15	max	-3.114	10	57.961	2	9.791	4	0	2	0	15	.167	3
524			min	-38.764	1	-27.122	3	-.74	10	0	3	-.055	1	-.338	2
525		16	max	-3.114	10	22.299	3	30.226	1	0	2	.006	5	.169	3
526			min	-38.764	1	-43.401	2	2.155	12	0	3	-.045	1	-.342	2
527		17	max	-3.114	10	71.72	3	55.564	1	0	2	.012	5	.141	3
528			min	-38.764	1	-144.762	2	3.265	12	0	3	-.02	1	-.287	2
529		18	max	-3.114	10	121.141	3	80.903	1	0	2	.025	4	.085	3
530			min	-38.764	1	-246.124	2	4.375	12	0	3	0	10	-.173	2
531		19	max	-3.114	10	170.562	3	106.241	1	0	2	.074	1	0	2
532			min	-38.764	1	-347.486	2	5.484	12	0	3	.006	12	0	3
533	M15	1	max	0	1	1.208	9	.086	3	0	9	0	9	0	1
534			min	-91.278	3	0	1	-.019	9	0	3	0	3	0	1
535		2	max	0	1	1.073	9	.086	3	0	9	0	9	0	1
536			min	-91.354	3	0	1	-.019	9	0	3	0	3	0	9
537		3	max	0	1	.939	9	.086	3	0	9	0	9	0	1
538			min	-91.429	3	0	1	-.019	9	0	3	0	3	0	9
539		4	max	0	1	.805	9	.086	3	0	9	0	9	0	1
540			min	-91.505	3	0	1	-.019	9	0	3	0	3	-.001	9
541		5	max	0	1	.671	9	.086	3	0	9	0	9	0	1



Company : Schletter, Inc.  
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Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-91.58	3	0	1	-.019	9	0	3	0	3	-.001	9
543		6	max	0	1	.537	9	.086	3	0	9	0	9	0	1
544			min	-91.656	3	0	1	-.019	9	0	3	0	3	-.002	9
545		7	max	0	1	.403	9	.086	3	0	9	0	3	0	1
546			min	-91.731	3	0	1	-.019	9	0	3	0	9	-.002	9
547		8	max	0	1	.268	9	.086	3	0	9	0	3	0	1
548			min	-91.807	3	0	1	-.019	9	0	3	0	9	-.002	9
549		9	max	0	1	.134	9	.086	3	0	9	0	3	0	1
550			min	-91.882	3	0	1	-.019	9	0	3	0	9	-.002	9
551		10	max	0	1	0	1	.086	3	0	9	0	3	0	1
552			min	-91.958	3	0	1	-.019	9	0	3	0	9	-.002	9
553		11	max	0	1	0	1	.086	3	0	9	0	3	0	1
554			min	-92.034	3	-.134	9	-.019	9	0	3	0	9	-.002	9
555		12	max	0	1	0	1	.086	3	0	9	0	3	0	1
556			min	-92.109	3	-.268	9	-.019	9	0	3	0	9	-.002	9
557		13	max	0	1	0	1	.086	3	0	9	0	3	0	1
558			min	-92.185	3	-.403	9	-.019	9	0	3	0	9	-.002	9
559		14	max	0	1	0	1	.086	3	0	9	0	3	0	1
560			min	-92.26	3	-.537	9	-.019	9	0	3	0	9	-.002	9
561		15	max	0	1	0	1	.086	3	0	9	0	3	0	1
562			min	-92.336	3	-.671	9	-.019	9	0	3	0	9	-.001	9
563		16	max	0	1	0	1	.086	3	0	9	0	3	0	1
564			min	-92.411	3	-.805	9	-.019	9	0	3	0	9	-.001	9
565		17	max	0	1	0	1	.086	3	0	9	0	3	0	1
566			min	-92.487	3	-.939	9	-.019	9	0	3	0	9	0	9
567		18	max	0	1	0	1	.086	3	0	9	0	3	0	1
568			min	-92.562	3	-1.073	9	-.019	9	0	3	0	9	0	9
569		19	max	0	1	0	1	.086	3	0	9	0	3	0	1
570			min	-92.638	3	-1.208	9	-.019	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.628	4	.337	4	0	3	0	3	0	1
572			min	-202.986	4	0	10	-.035	3	0	2	0	4	0	1
573		2	max	0	10	2.336	4	.303	4	0	3	0	3	0	10
574			min	-202.993	4	0	10	-.035	3	0	2	0	4	0	4
575		3	max	0	10	2.044	4	.268	4	0	3	0	3	0	10
576			min	-203	4	0	10	-.035	3	0	2	0	4	-.002	4
577		4	max	0	10	1.752	4	.234	4	0	3	0	3	0	10
578			min	-203.008	4	0	10	-.035	3	0	2	0	4	-.002	4
579		5	max	0	10	1.46	4	.2	4	0	3	0	3	0	10
580			min	-203.015	4	0	10	-.035	3	0	2	0	1	-.003	4
581		6	max	0	10	1.168	4	.166	4	0	3	0	3	0	10
582			min	-203.022	4	0	10	-.035	3	0	2	0	1	-.003	4
583		7	max	0	10	.876	4	.132	4	0	3	0	5	0	10
584			min	-203.029	4	0	10	-.035	3	0	2	0	1	-.004	4
585		8	max	0	10	.584	4	.098	4	0	3	0	5	0	10
586			min	-203.037	4	0	10	-.035	3	0	2	0	1	-.004	4
587		9	max	0	10	.292	4	.063	4	0	3	0	5	0	10
588			min	-203.044	4	0	10	-.035	3	0	2	0	1	-.004	4
589		10	max	0	10	0	1	.03	2	0	3	0	5	0	10
590			min	-203.051	4	0	1	-.035	3	0	2	0	1	-.004	4
591		11	max	.091	2	0	10	.03	2	0	3	0	5	0	10
592			min	-203.058	4	-.292	4	-.035	3	0	2	0	1	-.004	4
593		12	max	.191	2	0	10	.03	2	0	3	0	5	0	10
594			min	-203.066	4	-.584	4	-.042	5	0	2	0	1	-.004	4
595		13	max	.292	2	0	10	.03	2	0	3	0	5	0	10
596			min	-203.073	4	-.876	4	-.076	5	0	2	0	3	-.004	4
597		14	max	.393	2	0	10	.03	2	0	3	0	5	0	10
598			min	-203.08	4	-1.168	4	-.111	5	0	2	0	3	-.003	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.493	2	0	10	.03	2	0	3	0	5	0	10
600		min	-203.088	4	-1.46	4	-.145	5	0	2	0	3	-.003	4
601	16	max	.594	2	0	10	.03	2	0	3	0	2	0	10
602		min	-203.095	4	-1.752	4	-.179	5	0	2	0	3	-.002	4
603	17	max	.695	2	0	10	.03	2	0	3	0	2	0	10
604		min	-203.102	4	-2.044	4	-.213	5	0	2	0	3	-.002	4
605	18	max	.795	2	0	10	.03	2	0	3	0	2	0	10
606		min	-203.109	4	-2.336	4	-.247	5	0	2	0	5	0	4
607	19	max	.896	2	0	10	.03	2	0	3	0	2	0	1
608		min	-203.117	4	-2.628	4	-.282	5	0	2	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.011	2	.007	1	1.25e-3	5	NC	3	NC	2	
2			min	-.004	3	-.011	3	-.013	5	-6.159e-4	1	3958.882	2	5786.387	1	
3			2	max	.002	1	.01	2	.007	1	1.271e-3	5	NC	3	NC	2
4				min	-.004	3	-.011	3	-.012	5	-5.888e-4	1	4330.489	2	6212.26	1
5			3	max	.002	1	.009	2	.006	1	1.293e-3	5	NC	3	NC	2
6				min	-.003	3	-.01	3	-.012	5	-5.616e-4	1	4774.244	2	6716.88	1
7			4	max	.002	1	.008	2	.006	1	1.314e-3	5	NC	1	NC	2
8				min	-.003	3	-.01	3	-.012	5	-5.344e-4	1	5307.803	2	7318.769	1
9			5	max	.002	1	.007	2	.005	1	1.336e-3	5	NC	1	NC	2
10				min	-.003	3	-.009	3	-.012	5	-5.072e-4	1	5954.771	2	8042.676	1
11			6	max	.002	1	.006	2	.005	1	1.358e-3	5	NC	1	NC	2
12				min	-.003	3	-.009	3	-.011	5	-4.8e-4	1	6747.208	2	8922.238	1
13			7	max	.001	1	.005	2	.004	1	1.379e-3	5	NC	1	NC	1
14				min	-.003	3	-.008	3	-.011	5	-4.528e-4	1	7729.482	2	NC	1
15			8	max	.001	1	.005	2	.004	1	1.401e-3	5	NC	1	NC	1
16				min	-.002	3	-.008	3	-.011	5	-4.257e-4	1	8964.365	2	NC	1
17			9	max	.001	1	.004	2	.003	1	1.423e-3	5	NC	1	NC	1
18				min	-.002	3	-.007	3	-.01	5	-3.985e-4	1	NC	1	NC	1
19			10	max	.001	1	.003	2	.003	1	1.444e-3	5	NC	1	NC	1
20				min	-.002	3	-.007	3	-.009	5	-3.713e-4	1	NC	1	NC	1
21		11	max	0	1	.003	2	.002	1	1.466e-3	5	NC	1	NC	1	
22			min	-.002	3	-.006	3	-.009	5	-3.441e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	1.487e-3	5	NC	1	NC	1	
24			min	-.002	3	-.005	3	-.008	5	-3.169e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.002	1	1.509e-3	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.007	5	-2.897e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.001	1	1.531e-3	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.006	5	-2.626e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.552e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.005	5	-2.354e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.574e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.004	5	-2.082e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.596e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.003	5	-1.81e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.617e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-1.538e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.639e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.267e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	6.066e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-7.838e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.004	5	7.401e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-7.931e-4	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.008	5	8.737e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-8.023e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.012	5	1.007e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-8.116e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.016	5	1.141e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	0	1	-8.209e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.02	5	1.274e-4	1	NC	1	NC	1
50			min	0	2	-.005	3	0	1	-8.302e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.024	4	1.408e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-8.395e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.027	4	1.541e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-8.488e-4	5	NC	1	NC	1
55		9	max	.001	3	.001	2	.031	4	1.675e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	9	-8.581e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.035	4	1.809e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-8.674e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.038	4	1.942e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-8.767e-4	5	NC	1	NC	1
61		12	max	.002	3	.003	2	.042	4	2.076e-4	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	10	-8.86e-4	5	NC	1	NC	1
63		13	max	.002	3	.004	2	.045	4	2.209e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	10	-8.953e-4	5	NC	1	NC	1
65		14	max	.002	3	.005	2	.049	4	2.343e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	10	-9.046e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.052	4	2.476e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	10	-9.139e-4	5	8523.619	2	NC	1
69		16	max	.002	3	.006	2	.055	4	2.61e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	10	-9.232e-4	5	7235.804	2	NC	1
71		17	max	.002	3	.007	2	.059	4	2.743e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-9.325e-4	5	6237.12	2	NC	1
73		18	max	.002	3	.008	2	.062	4	2.877e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-9.418e-4	5	5453.925	2	NC	1
75		19	max	.002	3	.01	2	.065	4	3.01e-4	1	NC	3	NC	1
76			min	-.002	2	-.009	3	0	10	-9.511e-4	5	4834.303	2	NC	1
77	M4	1	max	.001	1	.013	2	0	10	5.201e-3	5	NC	1	NC	2
78			min	0	15	-.011	3	-.068	4	-4.832e-4	1	NC	1	282.822	4
79		2	max	.001	1	.012	2	0	10	5.201e-3	5	NC	1	NC	2
80			min	0	15	-.01	3	-.063	4	-4.832e-4	1	NC	1	308.296	4
81		3	max	.001	1	.011	2	0	10	5.201e-3	5	NC	1	NC	2
82			min	0	15	-.01	3	-.057	4	-4.832e-4	1	NC	1	338.615	4
83		4	max	.001	1	.01	2	0	10	5.201e-3	5	NC	1	NC	2
84			min	0	15	-.009	3	-.052	4	-4.832e-4	1	NC	1	375.054	4
85		5	max	.001	1	.01	2	0	10	5.201e-3	5	NC	1	NC	2
86			min	0	15	-.009	3	-.046	4	-4.832e-4	1	NC	1	419.351	4
87		6	max	.001	1	.009	2	0	10	5.201e-3	5	NC	1	NC	1
88			min	0	15	-.008	3	-.041	4	-4.832e-4	1	NC	1	473.923	4
89		7	max	0	1	.008	2	0	10	5.201e-3	5	NC	1	NC	1
90			min	0	15	-.007	3	-.036	4	-4.832e-4	1	NC	1	542.212	4
91		8	max	0	1	.008	2	0	10	5.201e-3	5	NC	1	NC	1
92			min	0	15	-.007	3	-.031	4	-4.832e-4	1	NC	1	629.258	4
93		9	max	0	1	.007	2	0	10	5.201e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.026	4	-4.832e-4	1	NC	1	742.685	4
95		10	max	0	1	.006	2	0	10	5.201e-3	5	NC	1	NC	1
96			min	0	15	-.006	3	-.022	4	-4.832e-4	1	NC	1	894.475	4
97		11	max	0	1	.006	2	0	10	5.201e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.018	4	-4.832e-4	1	NC	1	1104.364	4
99		12	max	0	1	.005	2	0	10	5.201e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.014	4	-4.832e-4	1	NC	1	1406.774	4
101		max	0	1	.004	2	0	10	5.201e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.01	4	-4.832e-4	1	NC	1	1866.273	4
103		max	0	1	.003	2	0	10	5.201e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.007	4	-4.832e-4	1	NC	1	2616.041	4
105		max	0	1	.003	2	0	10	5.201e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.005	4	-4.832e-4	1	NC	1	3968.957	4
107		max	0	1	.002	2	0	10	5.201e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.003	4	-4.832e-4	1	NC	1	6814.439	4
109		max	0	1	.001	2	0	10	5.201e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.001	4	-4.832e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	5.201e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-4.832e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	5.201e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-4.832e-4	1	NC	1	NC	1
115	M6	max	.007	1	.036	2	.003	1	1.348e-3	4	NC	3	NC	1
116		min	-.012	3	-.035	3	-.013	5	-2.028e-7	2	1163.36	2	7106.513	3
117		max	.007	1	.034	2	.003	1	1.369e-3	4	NC	3	NC	1
118		min	-.011	3	-.033	3	-.013	5	-1.694e-6	1	1245.767	2	7516.469	3
119		max	.006	1	.032	2	.002	1	1.389e-3	4	NC	3	NC	1
120		min	-.011	3	-.031	3	-.013	5	-4.88e-6	1	1340.309	2	8006.948	3
121		max	.006	1	.029	2	.002	1	1.41e-3	4	NC	3	NC	1
122		min	-.01	3	-.029	3	-.012	5	-8.067e-6	1	1449.406	2	8594.546	3
123		max	.006	1	.027	2	.002	1	1.431e-3	4	NC	3	NC	1
124		min	-.009	3	-.028	3	-.012	5	-1.125e-5	1	1576.177	2	9301.366	3
125		max	.005	1	.025	2	.002	1	1.451e-3	4	NC	3	NC	1
126		min	-.009	3	-.026	3	-.012	5	-1.444e-5	1	1724.703	2	NC	1
127		max	.005	1	.022	2	.002	1	1.472e-3	4	NC	3	NC	1
128		min	-.008	3	-.024	3	-.011	5	-1.763e-5	1	1900.426	2	NC	1
129		max	.004	1	.02	2	.001	1	1.493e-3	4	NC	3	NC	1
130		min	-.007	3	-.022	3	-.011	5	-2.081e-5	1	2110.766	2	NC	1
131		max	.004	1	.018	2	.001	1	1.513e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.01	5	-2.4e-5	1	2366.107	2	NC	1
133		max	.004	1	.016	2	.001	1	1.534e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.01	5	-2.719e-5	1	2681.442	2	NC	1
135		max	.003	1	.014	2	0	1	1.555e-3	4	NC	3	NC	1
136		min	-.005	3	-.016	3	-.009	5	-3.037e-5	1	3079.24	2	NC	1
137		max	.003	1	.012	2	0	1	1.575e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.008	5	-3.356e-5	1	3594.792	2	NC	1
139		max	.002	1	.01	2	0	1	1.596e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.007	5	-3.675e-5	1	4286.875	2	NC	1
141		max	.002	1	.008	2	0	1	1.617e-3	4	NC	3	NC	1
142		min	-.003	3	-.01	3	-.006	5	-3.993e-5	1	5261.237	2	NC	1
143		max	.002	1	.006	2	0	1	1.637e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.005	5	-4.312e-5	1	6729.282	2	NC	1
145		max	.001	1	.005	2	0	1	1.658e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.004	5	-4.631e-5	1	9184.155	2	NC	1
147		max	0	1	.003	2	0	1	1.679e-3	5	NC	1	NC	1
148		min	-.001	3	-.004	3	-.003	5	-4.949e-5	1	NC	1	NC	1
149		max	0	1	.001	2	0	1	1.7e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-5.268e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.722e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.587e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.655e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-8.233e-4	5	NC	1	NC	1
155		max	0	3	.002	2	.004	5	2.269e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-8.202e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.008	5	1.883e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-8.179e-4	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.012	5	1.497e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-8.156e-4	4	NC	1	NC	1
161		5	max	.002	3	.006	2	.017	5	1.112e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-8.133e-4	4	7835.427	2	NC	1
163		6	max	.002	3	.007	2	.021	5	2.81e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	-8.11e-4	4	6273.529	2	NC	1
165		7	max	.003	3	.009	2	.025	5	5.203e-5	3	NC	3	NC	1
166			min	-.003	2	-.012	3	0	1	-8.087e-4	4	5208.433	2	NC	1
167		8	max	.003	3	.01	2	.029	5	7.595e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-8.065e-4	4	4429.897	2	NC	1
169		9	max	.003	3	.012	2	.032	5	9.988e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-8.042e-4	4	3833.073	2	NC	1
171		10	max	.004	3	.014	2	.036	5	1.238e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	0	1	-8.019e-4	4	3359.805	2	NC	1
173		11	max	.004	3	.015	2	.04	4	1.477e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-7.996e-4	4	2975.16	2	NC	1
175		12	max	.005	3	.017	2	.043	4	1.717e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.001	1	-7.973e-4	4	2656.797	2	NC	1
177		13	max	.005	3	.019	2	.047	4	1.956e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.001	1	-7.951e-4	4	2389.673	2	NC	1
179		14	max	.005	3	.021	2	.05	4	2.195e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.001	1	-7.928e-4	4	2163.211	2	NC	1
181		15	max	.006	3	.023	2	.053	4	2.434e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.001	1	-7.905e-4	4	1969.707	2	NC	1
183		16	max	.006	3	.026	2	.056	4	2.674e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.001	1	-7.882e-4	4	1803.373	2	NC	1
185		17	max	.007	3	.028	2	.06	4	2.913e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.001	1	-7.859e-4	4	1659.755	2	NC	1
187		18	max	.007	3	.03	2	.063	4	3.152e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-7.836e-4	4	1535.359	2	NC	1
189		19	max	.008	3	.032	2	.066	4	3.391e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-7.814e-4	4	1427.399	2	NC	1
191	M8	1	max	.004	1	.042	2	.002	1	5.027e-3	4	NC	1	NC	1
192			min	0	15	-.035	3	-.069	4	-2.617e-4	3	NC	1	280.449	4
193		2	max	.004	1	.04	2	.001	1	5.027e-3	4	NC	1	NC	1
194			min	0	15	-.033	3	-.063	4	-2.617e-4	3	NC	1	305.709	4
195		3	max	.003	1	.038	2	.001	1	5.027e-3	4	NC	1	NC	1
196			min	0	15	-.031	3	-.058	4	-2.617e-4	3	NC	1	335.773	4
197		4	max	.003	1	.035	2	.001	1	5.027e-3	4	NC	1	NC	1
198			min	0	15	-.029	3	-.052	4	-2.617e-4	3	NC	1	371.906	4
199		5	max	.003	1	.033	2	.001	1	5.027e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.046	4	-2.617e-4	3	NC	1	415.831	4
201		6	max	.003	1	.03	2	0	1	5.027e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.041	4	-2.617e-4	3	NC	1	469.945	4
203		7	max	.003	1	.028	2	0	1	5.027e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.036	4	-2.617e-4	3	NC	1	537.661	4
205		8	max	.002	1	.026	2	0	1	5.027e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.031	4	-2.617e-4	3	NC	1	623.977	4
207		9	max	.002	1	.023	2	0	1	5.027e-3	4	NC	1	NC	1
208			min	0	15	-.019	3	-.026	4	-2.617e-4	3	NC	1	736.452	4
209		10	max	.002	1	.021	2	0	1	5.027e-3	4	NC	1	NC	1
210			min	0	15	-.017	3	-.022	4	-2.617e-4	3	NC	1	886.969	4
211		11	max	.002	1	.019	2	0	1	5.027e-3	4	NC	1	NC	1
212			min	0	15	-.015	3	-.018	4	-2.617e-4	3	NC	1	1095.098	4
213		12	max	.001	1	.016	2	0	1	5.027e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.007	4	4.424e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-7.913e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.01	4	2.05e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-8.554e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.013	4	-2.553e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-9.196e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.017	5	-1.029e-5	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-9.838e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.02	5	-1.17e-5	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-1.048e-3	4	NC	1	NC	1
281		8	max	0	3	.001	2	.024	5	-1.31e-5	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-1.112e-3	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.027	5	-1.451e-5	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.002	3	-1.176e-3	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.03	5	-1.591e-5	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.24e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.033	5	-1.732e-5	10	NC	1	NC	1
288			min	-.001	2	-.008	3	-.003	3	-1.305e-3	4	NC	1	NC	1
289		12	max	.002	3	.003	2	.036	5	-1.872e-5	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	1	-1.369e-3	4	NC	1	NC	1
291		13	max	.002	3	.004	2	.039	5	-2.013e-5	10	NC	1	NC	1
292			min	-.002	2	-.009	3	-.004	1	-1.433e-3	4	NC	1	NC	1
293		14	max	.002	3	.005	2	.043	5	-2.153e-5	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.004	1	-1.497e-3	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.046	5	-2.294e-5	10	NC	1	NC	2
296			min	-.002	2	-.009	3	-.005	1	-1.561e-3	4	8539.629	2	9630.367	1
297		16	max	.002	3	.006	2	.049	5	-2.434e-5	10	NC	1	NC	2
298			min	-.002	2	-.009	3	-.005	1	-1.626e-3	4	7248.16	2	8586.68	1
299		17	max	.002	3	.007	2	.052	5	-2.575e-5	10	NC	1	NC	2
300			min	-.002	2	-.009	3	-.006	1	-1.69e-3	4	6246.908	2	7748.932	1
301		18	max	.002	3	.008	2	.055	5	-2.715e-5	10	NC	1	NC	2
302			min	-.002	2	-.009	3	-.007	1	-1.754e-3	4	5461.875	2	7068.429	1
303		19	max	.003	3	.01	2	.058	5	-2.856e-5	10	NC	3	NC	2
304			min	-.002	2	-.009	3	-.007	1	-1.818e-3	4	4840.914	2	6510.59	1
305	M12	1	max	.001	1	.012	2	.006	1	6.004e-3	4	NC	1	NC	2
306			min	0	15	-.011	3	-.063	5	3.46e-5	10	NC	1	305.528	5
307		2	max	.001	1	.012	2	.005	1	6.004e-3	4	NC	1	NC	2
308			min	0	15	-.01	3	-.058	5	3.46e-5	10	NC	1	333.04	5
309		3	max	.001	1	.011	2	.005	1	6.004e-3	4	NC	1	NC	2
310			min	0	15	-.01	3	-.053	5	3.46e-5	10	NC	1	365.783	5
311		4	max	.001	1	.01	2	.004	1	6.004e-3	4	NC	1	NC	2
312			min	0	15	-.009	3	-.048	5	3.46e-5	10	NC	1	405.134	5
313		5	max	.001	1	.01	2	.004	1	6.004e-3	4	NC	1	NC	2
314			min	0	15	-.009	3	-.043	5	3.46e-5	10	NC	1	452.971	5
315		6	max	.001	1	.009	2	.004	1	6.004e-3	4	NC	1	NC	2
316			min	0	15	-.008	3	-.038	5	3.46e-5	10	NC	1	511.903	5
317		7	max	0	1	.008	2	.003	1	6.004e-3	4	NC	1	NC	2
318			min	0	15	-.007	3	-.033	5	3.46e-5	10	NC	1	585.648	5
319		8	max	0	1	.008	2	.003	1	6.004e-3	4	NC	1	NC	2
320			min	0	15	-.007	3	-.028	5	3.46e-5	10	NC	1	679.646	5
321		9	max	0	1	.007	2	.002	1	6.004e-3	4	NC	1	NC	2
322			min	0	15	-.006	3	-.024	5	3.46e-5	10	NC	1	802.13	5
323		10	max	0	1	.006	2	.002	1	6.004e-3	4	NC	1	NC	1
324			min	0	15	-.006	3	-.02	5	3.46e-5	10	NC	1	966.038	5
325		11	max	0	1	.006	2	.002	1	6.004e-3	4	NC	1	NC	1
326			min	0	15	-.005	3	-.016	5	3.46e-5	10	NC	1	1192.678	5
327		12	max	0	1	.005	2	.001	1	6.004e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.013	5	3.46e-5	10	NC	1	1519.219	5
329		max	0	1	.004	2	0	1	6.004e-3	4	NC	1	NC	1
330		min	0	15	-.004	3	-.01	5	3.46e-5	10	NC	1	2015.376	5
331		max	0	1	.003	2	0	1	6.004e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.007	5	3.46e-5	10	NC	1	2824.944	5
333		max	0	1	.003	2	0	1	6.004e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.005	5	3.46e-5	10	NC	1	4285.737	5
335		max	0	1	.002	2	0	1	6.004e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.003	5	3.46e-5	10	NC	1	7358.05	5
337		max	0	1	.001	2	0	1	6.004e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.001	5	3.46e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	6.004e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	3.46e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	6.004e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	3.46e-5	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.007	5	9.429e-3	2	NC	1	NC	1
344		min	-.01	2	-.023	2	-.003	1	-1.382e-2	3	NC	1	NC	1
345		max	.01	3	.017	3	.01	5	4.62e-3	2	NC	4	NC	1
346		min	-.01	2	-.014	2	-.006	1	-6.834e-3	3	5117.376	2	NC	1
347		max	.01	3	.007	3	.013	5	4.43e-4	5	NC	4	NC	2
348		min	-.01	2	-.005	2	-.007	1	-3.767e-4	1	2625.197	2	8638.561	5
349		max	.01	3	.003	2	.016	5	4.547e-4	5	NC	4	NC	2
350		min	-.01	2	-.002	3	-.008	1	-3.258e-4	1	1834.315	2	5423.057	5
351		max	.01	3	.009	2	.019	5	4.663e-4	5	NC	4	NC	2
352		min	-.01	2	-.009	3	-.008	1	-2.748e-4	1	1452.023	2	3863.579	5
353		max	.01	3	.015	2	.023	5	4.78e-4	5	NC	4	NC	2
354		min	-.01	2	-.014	3	-.008	1	-2.239e-4	1	1234.239	2	2957.637	5
355		max	.009	3	.02	2	.027	5	4.896e-4	5	NC	4	NC	1
356		min	-.01	2	-.019	3	-.007	1	-1.729e-4	1	1100.753	2	2373.588	5
357		max	.009	3	.023	2	.031	5	5.013e-4	5	NC	4	NC	1
358		min	-.01	2	-.022	3	-.006	1	-1.22e-4	1	1018.164	2	1970.434	5
359		max	.009	3	.025	2	.035	5	5.13e-4	5	NC	4	NC	1
360		min	-.01	2	-.023	3	-.004	1	-7.103e-5	1	970.887	2	1678.381	5
361		max	.009	3	.026	2	.039	5	5.246e-4	5	NC	4	NC	1
362		min	-.01	2	-.024	3	-.002	1	-2.007e-5	1	951.807	2	1439.484	4
363		max	.009	3	.026	2	.044	4	5.459e-4	4	NC	4	NC	1
364		min	-.01	2	-.023	3	0	1	8.36e-6	10	958.976	2	1258.929	4
365		max	.009	3	.024	2	.048	4	5.701e-4	4	NC	4	NC	1
366		min	-.01	2	-.021	3	0	10	1.209e-5	10	994.937	2	1120.009	4
367		max	.009	3	.021	2	.053	4	5.943e-4	4	NC	4	NC	1
368		min	-.01	2	-.018	3	0	10	1.581e-5	10	1068.123	2	1011.335	4
369		max	.009	3	.016	2	.057	4	6.185e-4	4	NC	4	NC	2
370		min	-.01	2	-.014	3	0	10	1.954e-5	10	1197.855	2	925.328	4
371		max	.009	3	.01	2	.061	4	6.427e-4	4	NC	4	NC	2
372		min	-.01	2	-.008	3	0	10	2.327e-5	10	1429.516	2	856.803	4
373		max	.009	3	.002	2	.065	4	9.16e-4	4	NC	4	NC	2
374		min	-.01	2	-.002	3	0	10	2.595e-5	10	1874.033	3	802.129	4
375		max	.009	3	.006	3	.068	4	7.118e-3	4	NC	4	NC	2
376		min	-.01	2	-.008	2	0	10	-8.76e-5	1	2719.951	3	758.775	4
377		max	.009	3	.014	3	.071	4	6.995e-3	2	NC	2	NC	1
378		min	-.01	2	-.019	2	0	10	-3.565e-3	3	5334.252	3	724.806	4
379		max	.009	3	.023	3	.073	4	1.412e-2	2	NC	1	NC	1
380		min	-.01	2	-.031	2	-.002	1	-7.254e-3	3	5718.141	2	699.724	4
381	M5	max	.03	3	.087	3	.007	5	1.424e-5	4	NC	1	NC	1
382		min	-.033	2	-.075	2	-.003	1	0	2	3598.275	3	NC	1
383		max	.03	3	.053	3	.01	5	2.218e-4	5	NC	4	NC	1
384		min	-.033	2	-.045	2	-.003	1	-4.572e-5	1	1561.816	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.021	3	.013	5	4.26e-4	5	NC	5	NC	1
386		min	-.033	2	-.017	2	-.003	1	-9.069e-5	1	800.842	2	NC	1
387	4	max	.03	3	.009	2	.016	5	4.464e-4	5	NC	5	NC	1
388		min	-.033	2	-.006	3	-.003	1	-8.652e-5	1	559.171	2	NC	1
389	5	max	.03	3	.031	2	.02	5	4.668e-4	5	NC	5	NC	1
390		min	-.033	2	-.028	3	-.003	1	-8.235e-5	1	442.325	2	9800.915	3
391	6	max	.03	3	.05	2	.024	5	4.872e-4	5	NC	5	NC	1
392		min	-.033	2	-.046	3	-.003	1	-7.818e-5	1	375.743	2	8827.321	3
393	7	max	.03	3	.065	2	.028	5	5.076e-4	5	NC	5	NC	1
394		min	-.033	2	-.06	3	-.003	1	-7.402e-5	1	334.916	2	8367.71	3
395	8	max	.029	3	.076	2	.033	5	5.28e-4	5	NC	5	NC	1
396		min	-.033	2	-.069	3	-.002	1	-6.985e-5	1	309.636	2	8247.756	3
397	9	max	.029	3	.084	2	.037	5	5.483e-4	5	NC	5	NC	1
398		min	-.033	2	-.075	3	-.002	1	-6.568e-5	1	295.139	2	8400.139	3
399	10	max	.029	3	.087	2	.042	5	5.687e-4	5	NC	5	NC	1
400		min	-.033	2	-.077	3	-.002	1	-6.151e-5	1	289.247	2	8814.036	3
401	11	max	.029	3	.086	2	.046	5	5.891e-4	5	NC	5	NC	1
402		min	-.033	2	-.074	3	-.002	1	-5.734e-5	1	291.364	2	9521.898	3
403	12	max	.029	3	.08	2	.051	5	6.095e-4	5	NC	5	NC	1
404		min	-.033	2	-.068	3	-.002	1	-5.317e-5	1	302.264	2	NC	1
405	13	max	.029	3	.069	2	.055	5	6.299e-4	5	NC	5	NC	1
406		min	-.033	2	-.058	3	-.002	1	-4.9e-5	1	324.523	2	NC	1
407	14	max	.029	3	.054	2	.059	5	6.502e-4	5	NC	5	NC	1
408		min	-.033	2	-.044	3	-.002	1	-4.484e-5	1	364.057	2	NC	1
409	15	max	.029	3	.033	2	.062	4	6.706e-4	5	NC	5	NC	1
410		min	-.033	2	-.027	3	-.002	1	-4.067e-5	1	434.788	2	NC	1
411	16	max	.029	3	.006	2	.066	4	9.378e-4	5	NC	5	NC	1
412		min	-.033	2	-.006	3	-.002	1	-3.999e-5	1	575.994	2	NC	1
413	17	max	.029	3	.019	3	.069	4	7.099e-3	4	NC	5	NC	1
414		min	-.033	2	-.026	2	-.002	1	-1.223e-4	1	855.501	3	NC	1
415	18	max	.029	3	.046	3	.071	4	3.642e-3	4	NC	4	NC	1
416		min	-.033	2	-.063	2	-.001	1	-6.253e-5	1	1677.693	3	NC	1
417	19	max	.029	3	.074	3	.074	4	3.697e-6	5	NC	3	NC	1
418		min	-.033	2	-.103	2	-.001	1	-9.318e-7	3	1708.002	2	NC	1
419	M9	1	max	.01	.027	3	.006	5	1.383e-2	3	NC	1	NC	1
420		min	-.01	2	-.023	2	-.003	1	-9.429e-3	2	NC	1	NC	1
421	2	max	.01	3	.016	3	.006	5	6.804e-3	3	NC	4	NC	1
422		min	-.01	2	-.014	2	0	9	-4.628e-3	2	5120.472	2	NC	1
423	3	max	.01	3	.006	3	.006	4	1.798e-4	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	3	-9.007e-5	3	2626.827	2	NC	1
425	4	max	.01	3	.003	2	.007	4	1.367e-4	1	NC	4	NC	1
426		min	-.01	2	-.002	3	-.001	3	-9.217e-5	3	1835.471	2	NC	1
427	5	max	.01	3	.009	2	.009	4	9.361e-5	1	NC	4	NC	1
428		min	-.01	2	-.009	3	-.002	3	-9.427e-5	3	1445.796	3	9839.376	3
429	6	max	.01	3	.015	2	.011	4	5.05e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.003	3	-9.637e-5	3	1232.392	3	7666.683	4
431	7	max	.01	3	.02	2	.014	4	4.102e-5	4	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-9.847e-5	3	1101.404	2	5189.696	4
433	8	max	.009	3	.023	2	.018	4	6.263e-5	5	NC	4	NC	1
434		min	-.01	2	-.022	3	-.004	3	-1.006e-4	3	1018.744	2	3771.405	4
435	9	max	.009	3	.025	2	.022	5	8.664e-5	5	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-1.027e-4	3	971.417	2	2883.5	4
437	10	max	.009	3	.026	2	.026	5	1.107e-4	5	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-1.219e-4	1	952.299	2	2290.297	4
439	11	max	.009	3	.026	2	.031	5	1.347e-4	5	NC	4	NC	1
440		min	-.01	2	-.023	3	-.005	3	-1.65e-4	1	959.441	2	1873.942	4
441	12	max	.009	3	.024	2	.036	5	1.587e-4	5	NC	4	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.01	2	-.021	3	-.005	1	-2.081e-4	1	995.379	2	1566.875	5
443		13	max	.009	3	.021	2	.041	5	1.827e-4	5	NC	4	NC	2
444			min	-.01	2	-.018	3	-.006	1	-2.512e-4	1	1068.542	2	1330.402	5
445		14	max	.009	3	.016	2	.047	5	2.067e-4	5	NC	4	NC	2
446			min	-.01	2	-.014	3	-.007	1	-2.943e-4	1	1198.241	2	1152.668	5
447		15	max	.009	3	.01	2	.052	5	2.307e-4	5	NC	4	NC	2
448			min	-.01	2	-.008	3	-.007	1	-3.374e-4	1	1429.824	2	1015.869	5
449		16	max	.009	3	.002	2	.058	5	5.236e-4	5	NC	4	NC	2
450			min	-.01	2	-.002	3	-.007	1	-3.702e-4	1	1864.206	3	908.582	5
451		17	max	.009	3	.006	3	.063	5	7.23e-3	4	NC	4	NC	2
452			min	-.01	2	-.008	2	-.006	1	-1.567e-4	1	2706.177	3	823.089	5
453		18	max	.009	3	.015	3	.068	5	3.648e-3	3	NC	2	NC	1
454			min	-.01	2	-.019	2	-.004	1	-7.001e-3	2	5307.77	3	748.866	4
455		19	max	.009	3	.023	3	.074	4	7.252e-3	3	NC	1	NC	1
456			min	-.01	2	-.031	2	-.001	1	-1.412e-2	2	5733.322	2	686.65	4
457	M13	1	max	.003	1	.027	3	.01	3	3.982e-3	3	NC	1	NC	1
458			min	-.006	5	-.023	2	-.01	2	-3.477e-3	2	NC	1	NC	1
459		2	max	.003	1	.12	3	.01	9	4.968e-3	3	NC	4	NC	2
460			min	-.006	5	-.087	2	-.005	10	-4.357e-3	2	1347.235	3	8488.804	1
461		3	max	.003	1	.197	3	.029	1	5.954e-3	3	NC	5	NC	2
462			min	-.006	5	-.14	2	-.004	10	-5.238e-3	2	737.236	3	3734.267	1
463		4	max	.003	1	.248	3	.044	1	6.94e-3	3	NC	5	NC	3
464			min	-.007	5	-.175	2	-.003	10	-6.118e-3	2	569.555	3	2586.233	1
465		5	max	.003	1	.266	3	.05	1	7.926e-3	3	NC	5	NC	3
466			min	-.007	5	-.189	2	-.005	5	-6.998e-3	2	526.669	3	2300.171	1
467		6	max	.003	1	.252	3	.045	1	8.912e-3	3	NC	5	NC	2
468			min	-.007	5	-.181	2	-.007	10	-7.878e-3	2	558.988	3	2525.164	1
469		7	max	.003	1	.213	3	.03	1	9.898e-3	3	NC	5	NC	2
470			min	-.007	5	-.156	2	-.01	10	-8.758e-3	2	677.869	3	3596.525	1
471		8	max	.003	1	.159	3	.024	3	1.088e-2	3	NC	5	NC	2
472			min	-.007	5	-.122	2	-.019	2	-9.639e-3	2	951.05	3	8237.139	9
473		9	max	.003	1	.11	3	.027	3	1.187e-2	3	NC	4	NC	1
474			min	-.007	5	-.09	2	-.028	2	-1.052e-2	2	1522.428	3	6682.299	2
475		10	max	.003	1	.087	3	.03	3	1.286e-2	3	NC	4	NC	4
476			min	-.007	5	-.075	2	-.033	2	-1.14e-2	2	2102.47	3	5412.282	2
477		11	max	.003	1	.11	3	.033	3	1.187e-2	3	NC	4	NC	1
478			min	-.007	5	-.09	2	-.028	2	-1.052e-2	2	1522.426	3	5454.063	3
479		12	max	.003	1	.159	3	.034	3	1.089e-2	3	NC	5	NC	2
480			min	-.007	5	-.122	2	-.019	2	-9.639e-3	2	951.049	3	5177.655	3
481		13	max	.003	1	.213	3	.034	3	9.904e-3	3	NC	5	NC	2
482			min	-.007	5	-.156	2	-.01	10	-8.759e-3	2	677.868	3	3579.345	1
483		14	max	.003	1	.252	3	.045	1	8.92e-3	3	NC	5	NC	2
484			min	-.007	5	-.181	2	-.007	10	-7.879e-3	2	558.987	3	2522.671	1
485		15	max	.003	1	.266	3	.05	1	7.936e-3	3	NC	5	NC	5
486			min	-.007	5	-.189	2	-.004	10	-6.999e-3	2	526.668	3	2303.882	1
487		16	max	.003	1	.248	3	.044	1	6.952e-3	3	NC	5	NC	4
488			min	-.007	5	-.175	2	-.003	10	-6.119e-3	2	569.555	3	2597.064	1
489		17	max	.003	1	.198	3	.029	1	5.968e-3	3	NC	5	NC	2
490			min	-.007	5	-.14	2	-.004	10	-5.239e-3	2	737.235	3	3762.627	1
491		18	max	.003	1	.121	3	.013	3	4.984e-3	3	NC	4	NC	2
492			min	-.007	5	-.087	2	-.005	10	-4.359e-3	2	1347.234	3	8604.946	1
493		19	max	.003	1	.027	3	.01	3	4.e-3	3	NC	1	NC	1
494			min	-.007	5	-.023	2	-.01	2	-3.479e-3	2	NC	1	NC	1
495	M16	1	max	.001	1	.023	3	.009	3	4.528e-3	2	NC	1	NC	1
496			min	-.074	4	-.031	2	-.01	2	-3.363e-3	3	NC	1	NC	1
497		2	max	.001	1	.073	3	.014	4	5.683e-3	2	NC	4	NC	2
498			min	-.074	4	-.127	2	-.005	10	-4.171e-3	3	1318.405	2	8492.395	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.001	1	.114	3	.029	1	6.837e-3	2	NC	5	NC	2
500		min	-.074	4	-.206	2	-.003	10	-4.979e-3	3	720.293	2	3735.225	1
501	4	max	.001	1	.142	3	.044	1	7.992e-3	2	NC	5	NC	10
502		min	-.074	4	-.258	2	-.003	10	-5.787e-3	3	554.852	2	2586.706	1
503	5	max	.001	1	.153	3	.05	1	9.146e-3	2	NC	5	NC	10
504		min	-.074	4	-.278	2	-.004	10	-6.596e-3	3	510.625	2	2300.542	1
505	6	max	.001	1	.149	3	.045	1	1.03e-2	2	NC	5	NC	2
506		min	-.074	4	-.265	2	-.007	10	-7.404e-3	3	537.67	2	2525.678	1
507	7	max	.001	1	.132	3	.031	3	1.146e-2	2	NC	5	NC	2
508		min	-.074	4	-.227	2	-.01	10	-8.212e-3	3	642.986	2	3598.037	1
509	8	max	.001	1	.108	3	.031	3	1.261e-2	2	NC	5	NC	2
510		min	-.074	4	-.174	2	-.019	2	-9.02e-3	3	878.587	2	5749.648	3
511	9	max	.001	1	.085	3	.03	3	1.376e-2	2	NC	4	NC	1
512		min	-.074	4	-.125	2	-.028	2	-9.828e-3	3	1337.496	2	5986.39	3
513	10	max	.001	1	.074	3	.029	3	1.492e-2	2	NC	4	NC	4
514		min	-.074	4	-.103	2	-.033	2	-1.064e-2	3	1761.059	2	5458.373	2
515	11	max	.001	1	.085	3	.027	3	1.376e-2	2	NC	4	NC	1
516		min	-.074	4	-.125	2	-.028	2	-9.826e-3	3	1337.496	2	6750.477	2
517	12	max	.001	1	.108	3	.026	3	1.261e-2	2	NC	5	NC	2
518		min	-.074	4	-.174	2	-.019	2	-9.016e-3	3	878.587	2	7500.344	3
519	13	max	.001	1	.132	3	.03	1	1.146e-2	2	NC	5	NC	2
520		min	-.074	4	-.227	2	-.01	10	-8.206e-3	3	642.986	2	3592.176	1
521	14	max	.002	1	.149	3	.045	1	1.03e-2	2	NC	5	NC	2
522		min	-.074	4	-.265	2	-.007	10	-7.396e-3	3	537.67	2	2529.289	1
523	15	max	.002	1	.153	3	.05	1	9.148e-3	2	NC	5	NC	3
524		min	-.074	4	-.278	2	-.004	10	-6.586e-3	3	510.625	2	2309.224	1
525	16	max	.002	1	.141	3	.044	1	7.994e-3	2	NC	5	NC	3
526		min	-.074	4	-.258	2	-.004	5	-5.776e-3	3	554.852	2	2603.088	1
527	17	max	.002	1	.114	3	.029	1	6.84e-3	2	NC	5	NC	2
528		min	-.074	4	-.206	2	-.005	5	-4.966e-3	3	720.293	2	3772.435	1
529	18	max	.002	1	.072	3	.011	3	5.685e-3	2	NC	4	NC	2
530		min	-.073	4	-.127	2	-.005	10	-4.156e-3	3	1318.405	2	8634.397	1
531	19	max	.002	1	.023	3	.009	3	4.531e-3	2	NC	1	NC	1
532		min	-.073	4	-.031	2	-.01	2	-3.346e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.197e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.863e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.007	4	8.959e-4	3	NC	1	NC	1
536		min	0	5	-.005	1	0	3	-7.028e-4	5	NC	1	NC	1
537	3	max	0	3	.001	5	.016	4	1.372e-3	3	NC	3	NC	1
538		min	-.001	5	-.01	1	-.004	3	-1.002e-3	2	7512.566	1	4856.669	4
539	4	max	0	3	.002	5	.025	4	1.848e-3	3	NC	5	NC	9
540		min	-.002	5	-.014	1	-.008	3	-1.472e-3	2	5154.059	1	3046.713	4
541	5	max	0	3	.003	5	.034	4	2.325e-3	3	NC	5	NC	9
542		min	-.002	5	-.019	1	-.013	3	-1.942e-3	2	4021.765	1	2239.264	4
543	6	max	0	3	.003	5	.042	4	2.801e-3	3	NC	5	NC	9
544		min	-.003	5	-.022	1	-.019	3	-2.413e-3	2	3384.74	1	1814.946	4
545	7	max	0	3	.004	5	.048	4	3.277e-3	3	NC	5	8026.553	9
546		min	-.004	5	-.025	1	-.025	3	-2.883e-3	2	3001.654	1	1578.277	4
547	8	max	0	3	.004	5	.052	4	3.753e-3	3	NC	5	6719.192	9
548		min	-.004	5	-.027	1	-.031	3	-3.353e-3	2	2771.745	1	1451.91	4
549	9	max	0	3	.005	5	.054	4	4.229e-3	3	NC	5	5854.034	9
550		min	-.005	5	-.028	1	-.036	3	-3.824e-3	2	2647.994	1	1402.781	4
551	10	max	0	3	.005	5	.054	4	4.706e-3	3	NC	5	5281.042	9
552		min	-.005	5	-.028	1	-.04	3	-4.294e-3	2	2608.845	1	1294.506	3
553	11	max	.001	3	.005	5	.051	4	5.182e-3	3	NC	5	4920.889	9
554		min	-.006	5	-.028	1	-.043	3	-4.764e-3	2	2647.994	1	1197.769	3
555	12	max	.001	3	.006	5	.046	4	5.658e-3	3	NC	5	4735.211	9



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.007	5	-.026	1	-.043	3	-5.235e-3	2	2771.745	1	1145.729	3
557		max	.001	3	.006	5	.039	4	6.134e-3	3	NC	5	4715.926	9
558		min	-.007	5	-.024	1	-.042	3	-5.705e-3	2	3001.654	1	1135.248	3
559		max	.001	3	.006	5	.03	4	6.611e-3	3	NC	5	4887.793	9
560		min	-.008	5	-.021	1	-.038	3	-6.175e-3	2	3384.74	1	1171.418	3
561		max	.001	3	.006	5	.024	1	7.087e-3	3	NC	5	7061.614	15
562		min	-.008	5	-.018	1	-.031	3	-6.646e-3	2	4021.765	1	1272.564	3
563		max	.002	3	.006	5	.017	1	7.563e-3	3	NC	5	NC	15
564		min	-.009	5	-.014	9	-.02	3	-7.116e-3	2	5154.059	1	1488.279	3
565		max	.002	3	.006	5	.006	1	8.039e-3	3	NC	3	NC	4
566		min	-.01	5	-.01	9	-.005	3	-7.586e-3	2	7512.566	1	1974.028	3
567		max	.002	3	.006	5	.014	3	8.515e-3	3	NC	1	NC	4
568		min	-.01	5	-.006	9	-.016	2	-8.057e-3	2	NC	1	3516.122	3
569		max	.002	3	.008	2	.037	3	8.992e-3	3	NC	1	NC	1
570		min	-.011	5	-.002	9	-.036	2	-8.527e-3	2	NC	1	NC	1
571	M16A	max	0	10	.002	2	.011	3	2.574e-3	3	NC	1	NC	1
572		min	-.004	4	-.004	4	-.011	2	-2.478e-3	2	NC	1	NC	1
573		max	0	10	-.003	10	.003	3	2.476e-3	3	NC	1	NC	1
574		min	-.004	4	-.013	4	-.004	2	-2.369e-3	2	8140.473	4	9977.315	3
575		max	0	10	-.006	12	.006	1	2.379e-3	3	NC	3	NC	4
576		min	-.004	4	-.022	4	-.009	5	-2.26e-3	2	4142.409	4	5654.56	3
577		max	0	10	-.008	12	.01	1	2.281e-3	3	NC	12	NC	9
578		min	-.003	4	-.03	4	-.017	5	-2.151e-3	2	2841.935	4	4308.888	3
579		max	0	10	-.01	12	.012	1	2.184e-3	3	8043.53	12	NC	9
580		min	-.003	4	-.038	4	-.026	5	-2.042e-3	2	2217.59	4	3104.412	5
581		max	0	10	-.012	12	.014	1	2.086e-3	3	6769.48	12	9610.901	9
582		min	-.003	4	-.044	4	-.035	5	-1.933e-3	2	1866.337	4	2252.782	5
583		max	0	10	-.013	12	.015	1	1.988e-3	3	6003.308	12	9299.89	9
584		min	-.003	4	-.049	4	-.044	5	-1.824e-3	2	1655.104	4	1797.336	5
585		max	0	10	-.014	12	.015	1	1.891e-3	3	5543.489	12	9369.146	9
586		min	-.002	4	-.052	4	-.051	5	-1.715e-3	2	1528.333	4	1537.444	5
587		max	0	10	-.015	12	.014	1	1.793e-3	3	5295.987	12	9774.428	9
588		min	-.002	4	-.054	4	-.056	5	-1.606e-3	2	1460.097	4	1390.72	5
589		max	0	10	-.015	12	.013	1	1.696e-3	3	5217.69	12	NC	9
590		min	-.002	4	-.055	4	-.059	5	-1.496e-3	2	1438.51	4	1320.324	5
591		max	0	10	-.015	12	.011	1	1.598e-3	3	5295.987	12	NC	9
592		min	-.002	4	-.054	4	-.059	5	-1.387e-3	2	1460.097	4	1311.073	5
593		max	0	10	-.014	12	.009	1	1.501e-3	3	5543.489	12	NC	9
594		min	-.002	4	-.051	4	-.057	5	-1.278e-3	2	1528.333	4	1361.436	5
595		max	0	10	-.013	12	.007	1	1.403e-3	3	6003.308	12	NC	2
596		min	-.001	4	-.047	4	-.052	5	-1.169e-3	2	1655.104	4	1482.974	5
597		max	0	10	-.011	12	.005	1	1.306e-3	3	6769.48	12	NC	1
598		min	-.001	4	-.042	4	-.045	5	-1.06e-3	2	1866.337	4	1706.884	5
599		max	0	10	-.01	12	.003	1	1.208e-3	3	8043.53	12	NC	1
600		min	0	4	-.035	4	-.037	5	-9.511e-4	2	2217.59	4	2105.219	5
601		max	0	10	-.008	12	.002	9	1.11e-3	3	NC	12	NC	1
602		min	0	4	-.027	4	-.027	5	-8.42e-4	2	2841.935	4	2859.007	5
603		max	0	10	-.005	12	0	9	1.013e-3	3	NC	3	NC	1
604		min	0	4	-.019	4	-.017	5	-7.33e-4	2	4142.409	4	4539.225	5
605		max	0	10	-.003	12	0	3	1.034e-3	4	NC	1	NC	1
606		min	0	4	-.01	4	-.008	5	-6.239e-4	2	8140.473	4	NC	1
607		max	0	1	0	1	0	1	1.103e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.148e-4	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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- Refer to manufacturer's product literature for hole cleaning and installation instructions.