



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.07	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	72 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	-1.496 k-ft
M_z =	-0.010 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	55%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.201 k-ft
M_z =	0.000 k-ft
P_n =	1.945 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	97%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.412 k-ft
P_n =	0.343 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	31%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

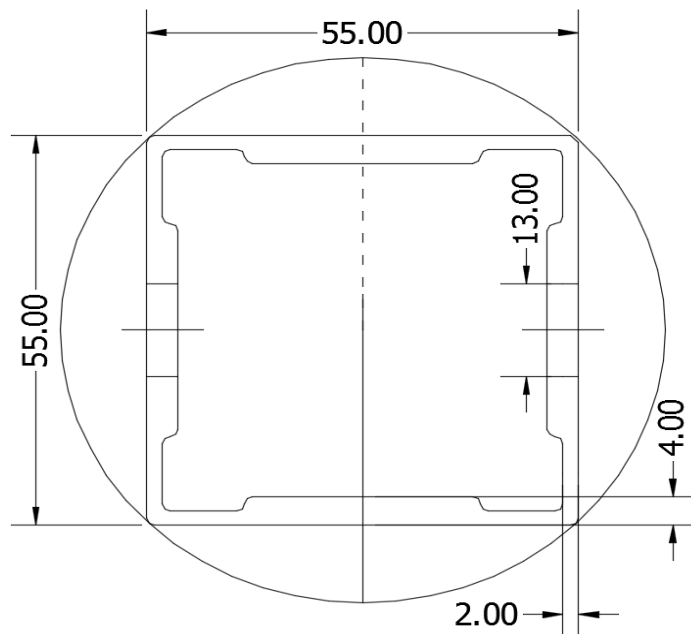
Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.012 k-ft
M_z =	0.000 k-ft
P_n =	2.750 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	47%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.012 k-ft
M_z =	0.000 k-ft
P_n =	3.186 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<u>37%</u>



5. FOUNDATION DESIGN CALCULATIONS

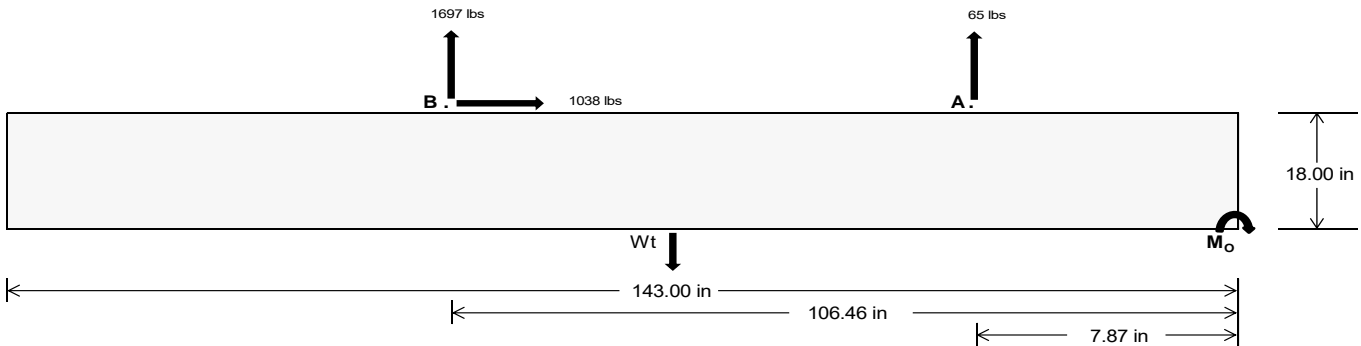
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>279.52</u>	<u>7064.99</u> k
Compressive Load =		<u>3108.19</u>	<u>5001.26</u> k
Lateral Load =		<u>284.43</u>	<u>4316.75</u> k
Moment (Weak Axis) =		<u>0.54</u>	<u>0.18</u> k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 199893.4$ in-lbs
Resisting Force Required = 2795.71 lbs
S.F. = 1.67
Weight Required = 4659.52 lbs
Minimum Width = 36 in
Weight Provided = 7775.63 lbs

Sliding

Force = 1038.25 lbs
Friction = 0.4
Weight Required = 2595.62 lbs
Resisting Weight = 7775.63 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 1038.25 lbs
Cohesion = 130 psf
Area = 35.75 ft²
Resisting = 3887.81 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 36in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 36in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(3 \text{ ft}) =$
36 in 37 in 38 in 39 in
7776 lbs 7992 lbs 8208 lbs 8424 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in
F_A	843 lbs	843 lbs	843 lbs	843 lbs	1334 lbs	1334 lbs	1334 lbs	1334 lbs	1541 lbs	1541 lbs	1541 lbs	1541 lbs	-130 lbs	-130 lbs	-130 lbs	-130 lbs
F_B	806 lbs	806 lbs	806 lbs	806 lbs	2249 lbs	2249 lbs	2249 lbs	2249 lbs	2204 lbs	2204 lbs	2204 lbs	2204 lbs	-3395 lbs	-3395 lbs	-3395 lbs	-3395 lbs
F_V	89 lbs	89 lbs	89 lbs	89 lbs	1853 lbs	1853 lbs	1853 lbs	1853 lbs	1448 lbs	1448 lbs	1448 lbs	1448 lbs	-2076 lbs	-2076 lbs	-2076 lbs	-2076 lbs
P_{total}	9425 lbs	9641 lbs	9857 lbs	10073 lbs	11359 lbs	11575 lbs	11791 lbs	12007 lbs	11521 lbs	11737 lbs	11953 lbs	12169 lbs	1141 lbs	1270 lbs	1400 lbs	1529 lbs
M	2254 lbs-ft	2254 lbs-ft	2254 lbs-ft	2254 lbs-ft	3301 lbs-ft	3301 lbs-ft	3301 lbs-ft	3301 lbs-ft	3922 lbs-ft	3922 lbs-ft	3922 lbs-ft	3922 lbs-ft	6084 lbs-ft	6084 lbs-ft	6084 lbs-ft	6084 lbs-ft
e	0.24 ft	0.23 ft	0.23 ft	0.22 ft	0.29 ft	0.29 ft	0.28 ft	0.27 ft	0.34 ft	0.33 ft	0.33 ft	0.32 ft	5.33 ft	4.79 ft	4.35 ft	3.98 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	231.9 psf	231.5 psf	231.1 psf	230.8 psf	271.2 psf	269.8 psf	268.4 psf	267.1 psf	267.0 psf	265.7 psf	264.4 psf	263.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	295.4 psf	293.3 psf	291.3 psf	289.4 psf	364.2 psf	360.3 psf	356.5 psf	352.9 psf	377.5 psf	373.2 psf	369.1 psf	365.2 psf	406.2 psf	235.1 psf	182.8 psf	158.4 psf

Maximum Bearing Pressure = 406 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

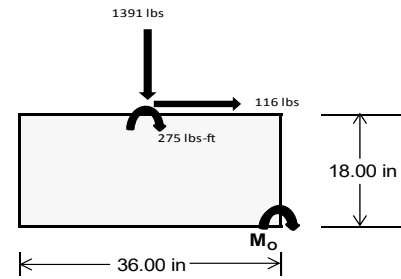
Overturning Check

$M_o = 1637.7 \text{ ft-lbs}$
 Resisting Force Required = 1091.81 lbs
 S.F. = 1.67
 Weight Required = 1819.68 lbs
 Minimum Width = **36 in**
 Weight Provided = 7775.63 lbs

A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	36 in			36 in			36 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	271 lbs	444 lbs	131 lbs	605 lbs	1391 lbs	498 lbs	129 lbs	130 lbs	-11 lbs
F_v	160 lbs	156 lbs	162 lbs	118 lbs	116 lbs	125 lbs	160 lbs	156 lbs	162 lbs
P_{total}	9897 lbs	10070 lbs	9757 lbs	9768 lbs	10554 lbs	9661 lbs	2943 lbs	2945 lbs	2804 lbs
M	607 lbs-ft	595 lbs-ft	614 lbs-ft	451 lbs-ft	449 lbs-ft	473 lbs-ft	607 lbs-ft	594 lbs-ft	609 lbs-ft
e	0.06 ft	0.06 ft	0.06 ft	0.05 ft	0.04 ft	0.05 ft	0.21 ft	0.20 ft	0.22 ft
$L/6$	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft
f_{min}	242.9 psf	248.4 psf	238.6 psf	248.0 psf	270.1 psf	243.8 psf	48.4 psf	49.2 psf	44.3 psf
f_{max}	310.8 psf	315.0 psf	307.3 psf	298.5 psf	320.3 psf	296.7 psf	116.3 psf	115.6 psf	112.5 psf



Maximum Bearing Pressure = 320 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 38in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

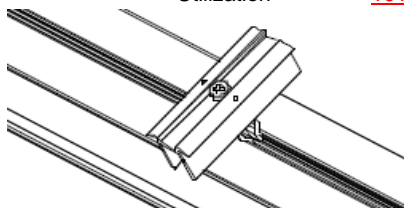
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

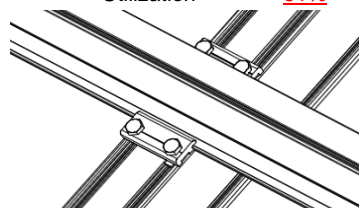
Fastening of Modules to Purlins

Maximum Uplifting Force =	1.230 k
Allowable Uplift =	1.214 k
Utilization =	101% $\leq 102\%$, $\Phi = 0.66$



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.649 k
Allowable Uplift =	4.357 k
Utilization =	61%



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.391 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	32%

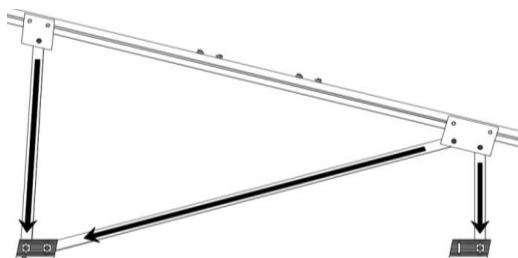
Rear Strut

Maximum Axial Load =	4.763 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	64%

Diagonal Strut

Maximum Axial Load =	2.911 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	39%

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

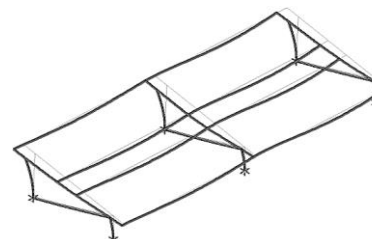
7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	60.93 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.219 in
	0.5 \leq 1.219, OK.

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 72 \text{ in}$$

$$J = 0.432$$

$$199.186$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 72$$

$$J = 0.432$$

$$126.67$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.88 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

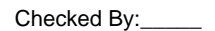
$$1.03 \text{ in}^2$$

$$P_{\max} = 9.14 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 130mph 30psf 6ft 7-05.r3d Page 18



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 130mph 30psf 6ft 7-05.r3d Page 19

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	43.841	4	732.459	2	-2.635	12	.013	3	.157	1	.934	2
20			min	1.662	10	-1319.521	3	-143.587	1	-.009	2	-.01	3	-1.496	3
21		11	max	35.974	4	604.28	2	-1.243	12	.013	3	.087	4	.488	2
22			min	1.662	10	-1081.702	3	-114.053	1	0	15	-.012	3	-.696	3
23		12	max	33.502	1	476.101	2	.611	3	.013	3	.047	4	.128	2
24			min	1.662	10	-843.883	3	-84.519	1	0	15	-.012	3	-.054	3
25		13	max	33.502	1	347.922	2	2.698	3	.013	3	.022	5	.43	3
26			min	1.662	10	-606.064	3	-54.985	1	0	15	-.041	1	-.146	2
27		14	max	33.502	1	219.743	2	4.785	3	.013	3	0	15	.754	3
28			min	1.662	10	-368.245	3	-38.592	4	0	15	-.068	1	-.336	2
29		15	max	33.502	1	91.564	2	6.873	3	.013	3	-.003	12	.921	3
30			min	-3.647	5	-130.425	3	-30.782	5	0	15	-.075	1	-.439	2
31		16	max	33.502	1	107.394	3	33.617	1	.013	3	0	3	.928	3
32			min	-11.514	5	-36.614	2	-28.664	5	0	15	-.063	1	-.458	2
33		17	max	33.502	1	345.213	3	63.151	1	.013	3	.007	3	.777	3
34			min	-19.381	5	-164.793	2	-26.546	5	0	15	-.067	4	-.391	2
35		18	max	33.502	1	583.032	3	92.685	1	.013	3	.021	1	.468	3
36			min	-27.248	5	-292.972	2	-24.428	5	0	15	-.077	5	-.238	2
37		19	max	33.502	1	820.851	3	122.219	1	.013	3	.093	1	0	2
38			min	-35.115	5	-421.151	2	-22.31	5	0	15	-.093	5	0	3
39	M14	1	max	27.776	4	534.414	2	-10.397	12	.016	3	.204	4	0	2
40			min	1.741	10	-700.059	3	-128.56	1	-.016	2	.007	10	0	3
41		2	max	25.376	1	406.235	2	-9.005	12	.016	3	.144	4	.406	3
42			min	1.741	10	-517.955	3	-99.026	1	-.016	2	-.001	10	-.314	2
43		3	max	25.376	1	278.056	2	-5.988	10	.016	3	.092	5	.691	3
44			min	1.741	10	-335.851	3	-76.419	4	-.016	2	-.014	1	-.542	2
45		4	max	25.376	1	149.877	2	-2.206	10	.016	3	.053	5	.854	3
46			min	-1.88	5	-153.748	3	-67.47	4	-.016	2	-.05	1	-.684	2
47		5	max	25.376	1	28.356	3	1.577	10	.016	3	.016	5	.896	3
48			min	-9.747	5	.071	15	-58.521	4	-.016	2	-.067	1	-.741	2
49		6	max	25.376	1	210.46	3	19.111	1	.016	3	-.005	12	.816	3
50			min	-17.614	5	-106.481	2	-52.739	5	-.016	2	-.064	1	-.713	2
51		7	max	25.376	1	392.564	3	48.645	1	.016	3	-.002	10	.615	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-2.508	10	1089.485	2	-3.787	12	.002	1	.182	4	1.321	2
134		min	-38.358	1	-568.843	3	-142.782	1	-.01	3	-.004	3	-.753	3
135	11	max	-2.508	10	896.304	2	-2.396	12	.01	3	.124	4	.659	2
136		min	-38.358	1	-470.313	3	-113.248	1	-.002	1	-.007	3	-.406	3
137	12	max	-2.508	10	703.124	2	-1.004	12	.01	3	.072	4	.126	2
138		min	-38.358	1	-371.784	3	-83.714	1	-.002	1	-.009	3	-.126	3
139	13	max	-2.508	10	509.943	2	.856	3	.01	3	.036	5	.089	3
140		min	-38.358	1	-273.254	3	-64.301	4	-.002	1	-.042	1	-.278	2
141	14	max	-2.508	10	316.763	2	2.943	3	.01	3	.002	5	.239	3
142		min	-38.358	1	-174.725	3	-55.352	4	-.002	1	-.068	1	-.554	2
143	15	max	-2.508	10	123.582	2	5.031	3	.01	3	-.003	12	.322	3
144		min	-45.334	4	-76.195	3	-47.497	5	-.002	1	-.075	1	-.701	2
145	16	max	-2.508	10	22.334	3	34.422	1	.01	3	0	12	.34	3
146		min	-53.201	4	-69.598	2	-45.379	5	-.002	1	-.075	4	-.719	2
147	17	max	-2.508	10	120.864	3	63.956	1	.01	3	.005	3	.293	3
148		min	-61.068	4	-262.779	2	-43.261	5	-.002	1	-.097	4	-.608	2
149	18	max	-2.508	10	219.393	3	93.49	1	.01	3	.023	1	.179	3
150		min	-68.935	4	-455.959	2	-41.143	5	-.002	1	-.119	5	-.368	2
151	19	max	-2.508	10	317.923	3	123.024	1	.01	3	.096	1	0	2
152		min	-76.802	4	-649.139	2	-39.025	5	-.002	1	-.145	5	0	5
153	M2	1	max	1117.033	2	2.063	.183	1	0	2	0	3	0	1
154		min	-1616.558	3	.501	15	-17.392	4	0	4	0	2	0	1
155	2	max	1117.563	2	1.992	4	.183	1	0	2	0	1	0	15
156		min	-1616.161	3	.485	15	-17.854	4	0	4	-.006	4	0	4
157	3	max	1118.092	2	1.921	4	.183	1	0	2	0	1	0	15
158		min	-1615.764	3	.468	15	-18.315	4	0	4	-.013	4	-.001	4
159	4	max	1118.621	2	1.85	4	.183	1	0	2	0	1	0	15
160		min	-1615.367	3	.451	15	-18.776	4	0	4	-.019	4	-.002	4
161	5	max	1119.151	2	1.779	4	.183	1	0	2	0	1	0	15
162		min	-1614.97	3	.435	15	-19.237	4	0	4	-.026	4	-.003	4
163	6	max	1119.68	2	1.708	4	.183	1	0	2	0	1	0	15
164		min	-1614.573	3	.418	15	-19.698	4	0	4	-.033	4	-.003	4
165	7	max	1120.209	2	1.637	4	.183	1	0	2	0	1	0	15
166		min	-1614.176	3	.401	15	-20.16	4	0	4	-.04	4	-.004	4
167	8	max	1120.738	2	1.566	4	.183	1	0	2	0	1	-.001	15
168		min	-1613.779	3	.384	15	-20.621	4	0	4	-.048	4	-.005	4
169	9	max	1121.268	2	1.495	4	.183	1	0	2	0	1	-.001	15
170		min	-1613.382	3	.361	12	-21.082	4	0	4	-.055	4	-.005	4
171	10	max	1121.797	2	1.424	4	.183	1	0	2	0	1	-.001	15
172		min	-1612.985	3	.333	12	-21.543	4	0	4	-.063	4	-.006	4
173	11	max	1122.326	2	1.353	4	.183	1	0	2	0	1	-.001	15
174		min	-1612.588	3	.305	12	-22.005	4	0	4	-.071	4	-.006	4
175	12	max	1122.856	2	1.282	4	.183	1	0	2	0	1	-.002	15
176		min	-1612.191	3	.278	12	-22.466	4	0	4	-.079	4	-.007	4
177	13	max	1123.385	2	1.211	4	.183	1	0	2	0	1	-.002	15
178		min	-1611.794	3	.25	12	-22.927	4	0	4	-.087	4	-.007	4
179	14	max	1123.914	2	1.14	4	.183	1	0	2	0	1	-.002	15
180		min	-1611.397	3	.222	12	-23.388	4	0	4	-.095	4	-.007	4
181	15	max	1124.443	2	1.069	4	.183	1	0	2	0	1	-.002	15
182		min	-1611	3	.195	12	-23.849	4	0	4	-.104	4	-.008	4
183	16	max	1124.973	2	.998	4	.183	1	0	2	0	1	-.002	12
184		min	-1610.603	3	.167	12	-24.311	4	0	4	-.112	4	-.008	4
185	17	max	1125.502	2	.936	2	.183	1	0	2	.001	1	-.002	12
186		min	-1610.206	3	.139	12	-24.772	4	0	4	-.121	4	-.009	4
187	18	max	1126.031	2	.881	2	.183	1	0	2	.001	1	-.002	12
188		min	-1609.809	3	.112	12	-25.233	4	0	4	-.13	4	-.009	4
189	19	max	1126.561	2	.825	2	.183	1	0	2	.001	1	-.002	12



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1609.412	3	.084	12	-25.694	4	0	4	-.139	4	-.009	4
191	M3	1	max	922.414	2	8.906	4	.854	4	0	10	0	1	.009	4
192			min	-1043.715	3	2.106	15	.011	10	0	4	-.015	4	.002	12
193		2	max	922.243	2	8.037	4	1.459	4	0	10	0	1	.005	2
194			min	-1043.842	3	1.902	15	.011	10	0	4	-.014	4	0	12
195		3	max	922.073	2	7.168	4	2.064	4	0	10	0	1	.002	2
196			min	-1043.97	3	1.697	15	.011	10	0	4	-.014	4	-.001	3
197		4	max	921.903	2	6.299	4	2.669	4	0	10	0	1	0	2
198			min	-1044.098	3	1.493	15	.011	10	0	4	-.012	5	-.003	3
199		5	max	921.732	2	5.43	4	3.274	4	0	10	0	1	0	15
200			min	-1044.226	3	1.289	15	.011	10	0	4	-.011	5	-.004	6
201		6	max	921.562	2	4.561	4	3.879	4	0	10	0	1	-.001	15
202			min	-1044.354	3	1.085	15	.011	10	0	4	-.009	5	-.007	6
203		7	max	921.392	2	3.692	4	4.484	4	0	10	0	1	-.002	15
204			min	-1044.481	3	.88	15	.011	10	0	4	-.008	5	-.009	6
205		8	max	921.221	2	2.823	4	5.089	4	0	10	0	1	-.002	15
206			min	-1044.609	3	.676	15	.011	10	0	4	-.005	5	-.01	6
207		9	max	921.051	2	1.954	4	5.695	4	0	10	0	1	-.003	15
208			min	-1044.737	3	.472	15	.011	10	0	4	-.003	5	-.011	6
209		10	max	920.881	2	1.086	4	6.3	4	0	10	0	1	-.003	15
210			min	-1044.865	3	.254	12	.011	10	0	4	0	5	-.012	6
211		11	max	920.71	2	.332	2	6.905	4	0	10	.003	4	-.003	15
212			min	-1044.992	3	-.143	3	.011	10	0	4	0	10	-.012	6
213		12	max	920.54	2	-.141	15	7.51	4	0	10	.007	4	-.003	15
214			min	-1045.12	3	-.653	6	.011	10	0	4	0	10	-.012	6
215		13	max	920.37	2	-.345	15	8.115	4	0	10	.01	4	-.003	15
216			min	-1045.248	3	-1.522	6	.011	10	0	4	0	10	-.012	6
217		14	max	920.199	2	-.549	15	8.72	4	0	10	.014	4	-.002	15
218			min	-1045.376	3	-2.391	6	.011	10	0	4	0	10	-.011	6
219		15	max	920.029	2	-.754	15	9.325	4	0	10	.019	4	-.002	15
220			min	-1045.503	3	-3.26	6	.011	10	0	4	0	10	-.009	6
221		16	max	919.858	2	-.958	15	9.93	4	0	10	.023	4	-.002	15
222			min	-1045.631	3	-4.129	6	.011	10	0	4	0	10	-.008	6
223		17	max	919.688	2	-1.162	15	10.535	4	0	10	.028	4	-.001	15
224			min	-1045.759	3	-4.998	6	.011	10	0	4	0	10	-.006	6
225		18	max	919.518	2	-1.366	15	11.14	4	0	10	.033	4	0	15
226			min	-1045.887	3	-5.867	6	.011	10	0	4	0	10	-.003	6
227		19	max	919.347	2	-1.571	15	11.745	4	0	10	.038	4	0	1
228			min	-1046.014	3	-6.736	6	.011	10	0	4	0	10	0	1
229	M4	1	max	846.287	1	0	1	-.414	10	0	1	.031	4	0	1
230			min	-100.729	5	0	1	-216.379	4	0	1	0	10	0	1
231		2	max	846.457	1	0	1	-.414	10	0	1	.006	4	0	1
232			min	-100.649	5	0	1	-216.527	4	0	1	0	10	0	1
233		3	max	846.627	1	0	1	-.414	10	0	1	0	12	0	1
234			min	-100.57	5	0	1	-216.674	4	0	1	-.019	4	0	1
235		4	max	846.798	1	0	1	-.414	10	0	1	0	10	0	1
236			min	-100.49	5	0	1	-216.822	4	0	1	-.044	4	0	1
237		5	max	846.968	1	0	1	-.414	10	0	1	0	10	0	1
238			min	-100.411	5	0	1	-216.969	4	0	1	-.069	4	0	1
239		6	max	847.139	1	0	1	-.414	10	0	1	0	10	0	1
240			min	-100.331	5	0	1	-217.117	4	0	1	-.094	4	0	1
241		7	max	847.309	1	0	1	-.414	10	0	1	0	10	0	1
242			min	-100.252	5	0	1	-217.265	4	0	1	-.118	4	0	1
243		8	max	847.479	1	0	1	-.414	10	0	1	0	10	0	1
244			min	-100.172	5	0	1	-217.412	4	0	1	-.143	4	0	1
245		9	max	847.65	1	0	1	-.414	10	0	1	0	10	0	1
246			min	-100.093	5	0	1	-217.56	4	0	1	-.168	4	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	847.82	1	0	1	-414	10	0	1	0	10	0	1
248			min	-100.013	5	0	1	-217.708	4	0	1	-.193	4	0	1
249		11	max	847.99	1	0	1	-414	10	0	1	0	10	0	1
250			min	-99.934	5	0	1	-217.855	4	0	1	-.218	4	0	1
251		12	max	848.161	1	0	1	-414	10	0	1	0	10	0	1
252			min	-99.854	5	0	1	-218.003	4	0	1	-.243	4	0	1
253		13	max	848.331	1	0	1	-414	10	0	1	0	10	0	1
254			min	-99.775	5	0	1	-218.151	4	0	1	-.268	4	0	1
255		14	max	848.501	1	0	1	-414	10	0	1	0	10	0	1
256			min	-99.695	5	0	1	-218.298	4	0	1	-.294	4	0	1
257		15	max	848.672	1	0	1	-414	10	0	1	0	10	0	1
258			min	-99.616	5	0	1	-218.446	4	0	1	-.319	4	0	1
259		16	max	848.842	1	0	1	-414	10	0	1	0	10	0	1
260			min	-99.536	5	0	1	-218.593	4	0	1	-.344	4	0	1
261		17	max	849.012	1	0	1	-414	10	0	1	0	10	0	1
262			min	-99.457	5	0	1	-218.741	4	0	1	-.369	4	0	1
263		18	max	849.183	1	0	1	-414	10	0	1	0	10	0	1
264			min	-99.377	5	0	1	-218.889	4	0	1	-.394	4	0	1
265		19	max	849.353	1	0	1	-414	10	0	1	0	10	0	1
266			min	-99.298	5	0	1	-219.036	4	0	1	-.419	4	0	1
267	M6	1	max	3176.561	2	2.294	2	0	1	0	1	0	4	0	1
268			min	-4762.718	3	.2	12	-17.58	4	0	5	0	1	0	1
269		2	max	3177.091	2	2.238	2	0	1	0	1	0	1	0	12
270			min	-4762.321	3	.172	12	-18.041	4	0	5	-.006	4	0	2
271		3	max	3177.62	2	2.183	2	0	1	0	1	0	1	0	12
272			min	-4761.924	3	.144	12	-18.503	4	0	5	-.013	4	-.002	2
273		4	max	3178.149	2	2.128	2	0	1	0	1	0	1	0	12
274			min	-4761.527	3	.117	12	-18.964	4	0	5	-.02	4	-.002	2
275		5	max	3178.679	2	2.072	2	0	1	0	1	0	1	0	12
276			min	-4761.13	3	.075	3	-19.425	4	0	5	-.027	4	-.003	2
277		6	max	3179.208	2	2.017	2	0	1	0	1	0	1	0	12
278			min	-4760.733	3	.034	3	-19.886	4	0	5	-.034	4	-.004	2
279		7	max	3179.737	2	1.962	2	0	1	0	1	0	1	0	12
280			min	-4760.336	3	-.008	3	-20.347	4	0	5	-.041	4	-.005	2
281		8	max	3180.266	2	1.906	2	0	1	0	1	0	1	0	3
282			min	-4759.939	3	-.049	3	-20.809	4	0	5	-.048	4	-.005	2
283		9	max	3180.796	2	1.851	2	0	1	0	1	0	1	0	3
284			min	-4759.542	3	-.091	3	-21.27	4	0	5	-.056	4	-.006	2
285		10	max	3181.325	2	1.795	2	0	1	0	1	0	1	0	3
286			min	-4759.145	3	-.132	3	-21.731	4	0	5	-.063	4	-.007	2
287		11	max	3181.854	2	1.74	2	0	1	0	1	0	1	0	3
288			min	-4758.748	3	-.174	3	-22.192	4	0	5	-.071	4	-.007	2
289		12	max	3182.384	2	1.685	2	0	1	0	1	0	1	0	3
290			min	-4758.351	3	-.215	3	-22.653	4	0	5	-.079	4	-.008	2
291		13	max	3182.913	2	1.629	2	0	1	0	1	0	1	0	3
292			min	-4757.954	3	-.257	3	-23.115	4	0	5	-.088	4	-.008	2
293		14	max	3183.442	2	1.574	2	0	1	0	1	0	1	0	3
294			min	-4757.557	3	-.298	3	-23.576	4	0	5	-.096	4	-.009	2
295		15	max	3183.971	2	1.519	2	0	1	0	1	0	1	0	3
296			min	-4757.16	3	-.34	3	-24.037	4	0	5	-.105	4	-.01	2
297		16	max	3184.501	2	1.463	2	0	1	0	1	0	1	0	3
298			min	-4756.763	3	-.381	3	-24.498	4	0	5	-.113	4	-.01	2
299		17	max	3185.03	2	1.408	2	0	1	0	1	0	1	0	3
300			min	-4756.366	3	-.423	3	-24.96	4	0	5	-.122	4	-.011	2
301		18	max	3185.559	2	1.353	2	0	1	0	1	0	1	0	3
302			min	-4755.969	3	-.464	3	-25.421	4	0	5	-.131	4	-.011	2
303		19	max	3186.089	2	1.297	2	0	1	0	1	0	1	0	3



Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2389.384	2	0	1	0	1	0	1	0	1	0	1
362			min	-216.169	3	0	1	-209.077	4	0	1	-.187	4	0	1
363		11	max	2389.554	2	0	1	0	1	0	1	0	1	0	1
364			min	-216.041	3	0	1	-209.225	4	0	1	-.211	4	0	1
365		12	max	2389.725	2	0	1	0	1	0	1	0	1	0	1
366			min	-215.913	3	0	1	-209.373	4	0	1	-.235	4	0	1
367		13	max	2389.895	2	0	1	0	1	0	1	0	1	0	1
368			min	-215.785	3	0	1	-209.52	4	0	1	-.259	4	0	1
369		14	max	2390.065	2	0	1	0	1	0	1	0	1	0	1
370			min	-215.657	3	0	1	-209.668	4	0	1	-.283	4	0	1
371		15	max	2390.236	2	0	1	0	1	0	1	0	1	0	1
372			min	-215.53	3	0	1	-209.815	4	0	1	-.307	4	0	1
373		16	max	2390.406	2	0	1	0	1	0	1	0	1	0	1
374			min	-215.402	3	0	1	-209.963	4	0	1	-.331	4	0	1
375		17	max	2390.576	2	0	1	0	1	0	1	0	1	0	1
376			min	-215.274	3	0	1	-210.111	4	0	1	-.355	4	0	1
377		18	max	2390.747	2	0	1	0	1	0	1	0	1	0	1
378			min	-215.146	3	0	1	-210.258	4	0	1	-.379	4	0	1
379		19	max	2390.917	2	0	1	0	1	0	1	0	1	0	1
380			min	-215.019	3	0	1	-210.406	4	0	1	-.404	4	0	1
381	M10	1	max	1117.033	2	1.989	6	-.009	10	0	1	0	4	0	1
382			min	-1616.558	3	.451	15	-17.511	4	0	5	0	3	0	1
383		2	max	1117.563	2	1.918	6	-.009	10	0	1	0	10	0	15
384			min	-1616.161	3	.434	15	-17.972	4	0	5	-.006	4	0	6
385		3	max	1118.092	2	1.847	6	-.009	10	0	1	0	10	0	15
386			min	-1615.764	3	.418	15	-18.434	4	0	5	-.013	4	-.001	6
387		4	max	1118.621	2	1.776	6	-.009	10	0	1	0	10	0	15
388			min	-1615.367	3	.401	15	-18.895	4	0	5	-.02	4	-.002	6
389		5	max	1119.151	2	1.705	6	-.009	10	0	1	0	10	0	15
390			min	-1614.97	3	.384	15	-19.356	4	0	5	-.026	4	-.003	6
391		6	max	1119.68	2	1.634	6	-.009	10	0	1	0	10	0	15
392			min	-1614.573	3	.367	15	-19.817	4	0	5	-.033	4	-.003	6
393		7	max	1120.209	2	1.563	6	-.009	10	0	1	0	10	0	15
394			min	-1614.176	3	.351	15	-20.279	4	0	5	-.041	4	-.004	6
395		8	max	1120.738	2	1.492	6	-.009	10	0	1	0	10	0	15
396			min	-1613.779	3	.334	15	-20.74	4	0	5	-.048	4	-.004	6
397		9	max	1121.268	2	1.421	6	-.009	10	0	1	0	10	-.001	15
398			min	-1613.382	3	.317	15	-21.201	4	0	5	-.056	4	-.005	6
399		10	max	1121.797	2	1.35	6	-.009	10	0	1	0	10	-.001	15
400			min	-1612.985	3	.301	15	-21.662	4	0	5	-.063	4	-.005	6
401		11	max	1122.326	2	1.279	6	-.009	10	0	1	0	10	-.001	15
402			min	-1612.588	3	.284	15	-22.123	4	0	5	-.071	4	-.006	6
403		12	max	1122.856	2	1.213	2	-.009	10	0	1	0	10	-.001	15
404			min	-1612.191	3	.267	15	-22.585	4	0	5	-.079	4	-.006	6
405		13	max	1123.385	2	1.157	2	-.009	10	0	1	0	10	-.002	15
406			min	-1611.794	3	.25	12	-23.046	4	0	5	-.087	4	-.007	6
407		14	max	1123.914	2	1.102	2	-.009	10	0	1	0	10	-.002	15
408			min	-1611.397	3	.222	12	-23.507	4	0	5	-.096	4	-.007	6
409		15	max	1124.443	2	1.047	2	-.009	10	0	1	0	10	-.002	15
410			min	-1611	3	.195	12	-23.968	4	0	5	-.104	4	-.007	6
411		16	max	1124.973	2	.991	2	-.009	10	0	1	0	10	-.002	15
412			min	-1610.603	3	.167	12	-24.43	4	0	5	-.113	4	-.008	6
413		17	max	1125.502	2	.936	2	-.009	10	0	1	0	10	-.002	15
414			min	-1610.206	3	.139	12	-24.891	4	0	5	-.122	4	-.008	6
415		18	max	1126.031	2	.881	2	-.009	10	0	1	0	10	-.002	15
416			min	-1609.809	3	.112	12	-25.352	4	0	5	-.131	4	-.008	6
417		19	max	1126.561	2	.825	2	-.009	10	0	1	0	10	-.002	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

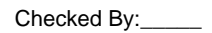
Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1609.412	3	.084	12	-25.813	4	0	5	-.14	4	-.009	6
419	M11	1	max	922.414	2	8.849	6	.75	4	0	1	0	10	.009	6
420			min	-1043.715	3	2.067	15	-.163	1	0	4	-.015	4	.002	15
421		2	max	922.243	2	7.98	6	1.355	4	0	1	0	10	.005	2
422			min	-1043.842	3	1.863	15	-.163	1	0	4	-.015	4	0	12
423		3	max	922.073	2	7.111	6	1.96	4	0	1	0	10	.002	2
424			min	-1043.97	3	1.659	15	-.163	1	0	4	-.014	4	-.001	3
425		4	max	921.903	2	6.242	6	2.565	4	0	1	0	10	0	2
426			min	-1044.098	3	1.455	15	-.163	1	0	4	-.013	4	-.003	3
427		5	max	921.732	2	5.373	6	3.17	4	0	1	0	10	-.001	15
428			min	-1044.226	3	1.25	15	-.163	1	0	4	-.011	4	-.005	4
429		6	max	921.562	2	4.505	6	3.775	4	0	1	0	10	-.002	15
430			min	-1044.354	3	1.046	15	-.163	1	0	4	-.01	4	-.007	4
431		7	max	921.392	2	3.636	6	4.38	4	0	1	0	10	-.002	15
432			min	-1044.481	3	.842	15	-.163	1	0	4	-.008	4	-.009	4
433		8	max	921.221	2	2.767	6	4.986	4	0	1	0	10	-.003	15
434			min	-1044.609	3	.638	15	-.163	1	0	4	-.006	4	-.01	4
435		9	max	921.051	2	1.898	6	5.591	4	0	1	0	10	-.003	15
436			min	-1044.737	3	.433	15	-.163	1	0	4	-.003	4	-.012	4
437		10	max	920.881	2	1.029	6	6.196	4	0	1	0	10	-.003	15
438			min	-1044.865	3	.229	15	-.163	1	0	4	0	1	-.012	4
439		11	max	920.71	2	.332	2	6.801	4	0	1	.003	5	-.003	15
440			min	-1044.992	3	-.143	3	-.163	1	0	4	0	1	-.012	4
441		12	max	920.54	2	-.179	15	7.406	4	0	1	.006	5	-.003	15
442			min	-1045.12	3	-.71	4	-.163	1	0	4	0	1	-.012	4
443		13	max	920.37	2	-.384	15	8.011	4	0	1	.01	5	-.003	15
444			min	-1045.248	3	-1.579	4	-.163	1	0	4	-.001	1	-.012	4
445		14	max	920.199	2	-.588	15	8.616	4	0	1	.014	5	-.003	15
446			min	-1045.376	3	-2.448	4	-.163	1	0	4	-.001	1	-.011	4
447		15	max	920.029	2	-.792	15	9.221	4	0	1	.018	5	-.002	15
448			min	-1045.503	3	-3.317	4	-.163	1	0	4	-.001	1	-.01	4
449		16	max	919.858	2	-.996	15	9.826	4	0	1	.022	5	-.002	15
450			min	-1045.631	3	-4.186	4	-.163	1	0	4	-.001	1	-.008	4
451		17	max	919.688	2	-1.201	15	10.431	4	0	1	.027	5	-.001	15
452			min	-1045.759	3	-5.054	4	-.163	1	0	4	-.001	1	-.006	4
453		18	max	919.518	2	-1.405	15	11.036	4	0	1	.032	5	0	15
454			min	-1045.887	3	-5.923	4	-.163	1	0	4	-.001	1	-.003	4
455		19	max	919.347	2	-1.609	15	11.641	4	0	1	.038	5	0	1
456			min	-1046.014	3	-6.792	4	-.163	1	0	4	-.002	1	0	1
457	M12	1	max	846.287	1	0	1	6.115	1	0	1	.03	5	0	1
458			min	-35.83	3	0	1	-212.453	4	0	1	-.001	1	0	1
459		2	max	846.457	1	0	1	6.115	1	0	1	.006	5	0	1
460			min	-35.702	3	0	1	-212.6	4	0	1	0	1	0	1
461		3	max	846.627	1	0	1	6.115	1	0	1	0	1	0	1
462			min	-35.574	3	0	1	-212.748	4	0	1	-.019	4	0	1
463		4	max	846.798	1	0	1	6.115	1	0	1	0	1	0	1
464			min	-35.446	3	0	1	-212.895	4	0	1	-.043	4	0	1
465		5	max	846.968	1	0	1	6.115	1	0	1	.002	1	0	1
466			min	-35.319	3	0	1	-213.043	4	0	1	-.068	4	0	1
467		6	max	847.139	1	0	1	6.115	1	0	1	.002	1	0	1
468			min	-35.191	3	0	1	-213.191	4	0	1	-.092	4	0	1
469		7	max	847.309	1	0	1	6.115	1	0	1	.003	1	0	1
470			min	-35.063	3	0	1	-213.338	4	0	1	-.117	4	0	1
471		8	max	847.479	1	0	1	6.115	1	0	1	.004	1	0	1
472			min	-34.935	3	0	1	-213.486	4	0	1	-.141	4	0	1
473		9	max	847.65	1	0	1	6.115	1	0	1	.004	1	0	1
474			min	-34.808	3	0	1	-213.634	4	0	1	-.166	4	0	1







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	695.602	3	52.279	2	73.244	4	0	3	0	1	1.21	3
590		min	-348.841	2	.014	15	3.396	10	0	9	-.071	4	-1.53	2
591	11	max	696.234	3	50.82	2	74.704	4	0	3	.037	1	1.189	3
592		min	-347.999	2	-1.732	6	3.396	10	0	9	-.034	5	-1.562	2
593	12	max	712.269	3	461.443	3	137.724	4	0	3	-.003	10	1.048	3
594		min	-297.229	2	-699.872	2	2.11	10	0	2	-.231	4	-1.39	2
595	13	max	712.901	3	460.349	3	139.184	4	0	3	-.002	10	.762	3
596		min	-296.386	2	-701.331	2	2.11	10	0	2	-.146	4	-.955	2
597	14	max	713.533	3	459.255	3	140.644	4	0	3	0	10	.477	3
598		min	-295.544	2	-702.79	2	2.11	10	0	2	-.059	4	-.519	2
599	15	max	714.165	3	458.16	3	142.104	4	0	3	.029	4	.192	3
600		min	-294.702	2	-704.249	2	2.11	10	0	2	0	10	-.089	1
601	16	max	714.796	3	457.066	3	143.565	4	0	3	.118	4	.355	2
602		min	-293.859	2	-705.708	2	2.11	10	0	2	.002	10	-.092	3
603	17	max	715.428	3	455.972	3	145.025	4	0	3	.207	4	.794	2
604		min	-293.017	2	-707.168	2	2.11	10	0	2	.003	10	-.375	3
605	18	max	-9.159	12	651.33	2	38.394	1	0	4	.218	4	.404	2
606		min	-123.863	1	-317.008	3	-62.822	5	0	3	.005	10	-.187	3
607	19	max	-8.738	12	649.87	2	38.394	1	0	4	.185	4	.01	3
608		min	-123.021	1	-318.103	3	-61.362	5	0	3	.006	10	-.002	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.241	2	.013	3	1.664e-2	2	NC	1	NC	1
2			min	-.52	4	-.086	3	-.008	2	-5.966e-3	3	NC	1	NC	1
3		2	max	0	1	.208	2	.015	3	1.717e-2	2	NC	4	NC	1
4			min	-.52	4	.005	15	-.006	2	-5.221e-3	3	1456.603	3	NC	1
5		3	max	0	1	.183	2	.018	3	1.769e-2	2	NC	4	NC	2
6			min	-.52	4	.004	15	-.008	5	-4.476e-3	3	792.593	3	8804.974	1
7		4	max	0	1	.171	2	.022	3	1.822e-2	2	NC	4	NC	2
8			min	-.52	4	.004	15	-.007	5	-3.731e-3	3	606.18	3	6180.419	1
9		5	max	0	1	.176	2	.026	3	1.874e-2	2	NC	4	NC	2
10			min	-.52	4	.004	15	-.007	10	-2.986e-3	3	551.402	3	5557.701	1
11		6	max	0	1	.196	2	.029	3	1.927e-2	2	NC	4	NC	2
12			min	-.52	4	.004	15	-.009	10	-2.241e-3	3	569.732	3	6181.412	1
13		7	max	0	1	.227	2	.032	3	1.979e-2	2	NC	2	NC	2
14			min	-.52	4	.004	15	-.012	2	-1.495e-3	3	659.916	3	7504.382	3
15		8	max	0	1	.263	2	.034	3	2.032e-2	2	NC	1	NC	1
16			min	-.52	4	.005	15	-.018	2	-7.502e-4	3	852.148	3	6768.78	3
17		9	max	0	1	.295	2	.035	3	2.084e-2	2	NC	4	NC	1
18			min	-.52	4	.005	15	-.023	2	-5.063e-6	3	1179.156	3	6398.743	3
19		10	max	0	1	.309	2	.036	3	2.137e-2	2	NC	4	NC	1
20			min	-.52	4	.006	15	-.026	2	3.727e-4	15	1436.307	3	6288.186	3
21		11	max	0	10	.295	2	.035	3	2.084e-2	2	NC	4	NC	1
22			min	-.52	4	.005	15	-.023	2	-5.063e-6	3	1179.156	3	6398.743	3
23		12	max	0	10	.263	2	.034	3	2.032e-2	2	NC	1	NC	1
24			min	-.52	4	.005	15	-.018	2	-7.502e-4	3	852.148	3	6768.78	3
25		13	max	0	10	.227	2	.032	3	1.979e-2	2	NC	2	NC	2
26			min	-.52	4	.004	15	-.012	2	-1.495e-3	3	659.916	3	7504.382	3
27		14	max	0	10	.196	2	.029	3	1.927e-2	2	NC	4	NC	2
28			min	-.52	4	.003	15	-.009	10	-2.241e-3	3	569.732	3	6181.412	1
29		15	max	0	10	.176	2	.026	3	1.874e-2	2	NC	4	NC	2
30			min	-.52	4	.003	15	-.007	10	-2.986e-3	3	551.402	3	5557.701	1
31		16	max	0	10	.171	2	.022	3	1.822e-2	2	NC	4	NC	2
32			min	-.52	4	.003	15	-.006	10	-3.731e-3	3	606.18	3	6180.419	1



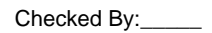
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	10	.183	2	.018	3	1.769e-2	2	NC	4	NC	2
34		min	-.52	4	.003	15	-.005	10	-4.476e-3	3	792.593	3	8410.066	4
35	18	max	0	10	.208	2	.015	3	1.717e-2	2	NC	4	NC	1
36		min	-.52	4	.004	15	-.006	2	-5.221e-3	3	1456.603	3	NC	1
37	19	max	0	10	.241	2	.013	3	1.664e-2	2	NC	1	NC	1
38		min	-.52	4	-.086	3	-.008	2	-5.966e-3	3	NC	1	NC	1
39	M14	1	max	0	.528	3	.011	3	8.881e-3	2	NC	1	NC	1
40		min	-.395	4	-.702	2	-.008	2	-7.714e-3	3	NC	1	NC	1
41	2	max	0	1	.671	3	.013	3	9.839e-3	2	NC	5	NC	1
42		min	-.395	4	-.842	2	-.01	5	-8.653e-3	3	1002.509	3	NC	1
43	3	max	0	1	.801	3	.015	3	1.08e-2	2	NC	5	NC	1
44		min	-.395	4	-.972	2	-.013	5	-9.592e-3	3	526.861	3	9541.212	5
45	4	max	0	1	.907	3	.018	3	1.176e-2	2	NC	5	NC	2
46		min	-.395	4	-1.083	2	-.01	5	-1.053e-2	3	377.382	2	7820.888	1
47	5	max	0	1	.983	3	.022	3	1.271e-2	2	NC	5	NC	2
48		min	-.395	4	-1.17	2	-.006	10	-1.147e-2	3	307.223	2	6682.618	1
49	6	max	0	1	1.027	3	.025	3	1.367e-2	2	NC	5	NC	2
50		min	-.395	4	-1.232	2	-.008	10	-1.241e-2	3	271.722	2	7186.421	1
51	7	max	0	1	1.043	3	.028	3	1.463e-2	2	NC	5	NC	1
52		min	-.395	4	-1.268	2	-.011	2	-1.335e-2	3	254.287	2	8745.427	3
53	8	max	0	1	1.038	3	.03	3	1.559e-2	2	NC	5	NC	1
54		min	-.395	4	-1.283	2	-.016	2	-1.429e-2	3	247.537	2	7764.842	3
55	9	max	0	1	1.023	3	.031	3	1.655e-2	2	NC	5	NC	1
56		min	-.395	4	-1.285	2	-.021	2	-1.523e-2	3	246.869	2	7272.978	3
57	10	max	0	1	1.014	3	.032	3	1.75e-2	2	NC	5	NC	1
58		min	-.395	4	-1.283	2	-.023	2	-1.617e-2	3	247.827	2	7124.48	3
59	11	max	0	10	1.023	3	.031	3	1.655e-2	2	NC	5	NC	1
60		min	-.395	4	-1.285	2	-.021	2	-1.523e-2	3	246.869	2	7272.978	3
61	12	max	0	10	1.038	3	.03	3	1.559e-2	2	NC	5	NC	1
62		min	-.395	4	-1.283	2	-.016	2	-1.429e-2	3	247.537	2	7764.842	3
63	13	max	0	10	1.043	3	.028	3	1.463e-2	2	NC	5	NC	1
64		min	-.395	4	-1.268	2	-.011	2	-1.335e-2	3	254.287	2	8745.427	3
65	14	max	0	10	1.027	3	.025	3	1.367e-2	2	NC	5	NC	2
66		min	-.395	4	-1.232	2	-.008	10	-1.241e-2	3	271.722	2	7186.421	1
67	15	max	0	10	.983	3	.022	3	1.271e-2	2	NC	5	NC	2
68		min	-.395	4	-1.17	2	-.006	10	-1.147e-2	3	307.223	2	6682.618	1
69	16	max	0	10	.907	3	.019	4	1.176e-2	2	NC	5	NC	2
70		min	-.395	4	-1.083	2	-.005	10	-1.053e-2	3	377.382	2	7453.226	4
71	17	max	0	10	.801	3	.019	4	1.08e-2	2	NC	5	NC	1
72		min	-.395	4	-.972	2	-.005	10	-9.592e-3	3	526.861	3	7160.298	4
73	18	max	0	10	.671	3	.013	4	9.839e-3	2	NC	5	NC	1
74		min	-.395	4	-.842	2	-.006	2	-8.653e-3	3	1002.509	3	NC	1
75	19	max	0	10	.528	3	.011	3	8.881e-3	2	NC	1	NC	1
76		min	-.395	4	-.702	2	-.008	2	-7.714e-3	3	NC	1	NC	1
77	M15	1	max	0	.539	3	.011	3	6.646e-3	3	NC	1	NC	1
78		min	-.327	4	-.7	2	-.007	2	-9.27e-3	2	NC	1	NC	1
79	2	max	0	10	.655	3	.012	3	7.44e-3	3	NC	5	NC	1
80		min	-.327	4	-.863	2	-.015	5	-1.028e-2	2	881.452	2	8323.596	5
81	3	max	0	10	.763	3	.014	3	8.235e-3	3	NC	5	NC	1
82		min	-.327	4	-1.011	2	-.02	5	-1.129e-2	2	462.057	2	6631.33	5
83	4	max	0	10	.856	3	.017	3	9.029e-3	3	NC	5	NC	2
84		min	-.327	4	-1.134	2	-.015	5	-1.23e-2	2	331.851	2	7744.607	1
85	5	max	0	10	.93	3	.02	3	9.823e-3	3	NC	5	NC	2
86		min	-.327	4	-1.223	2	-.006	5	-1.33e-2	2	274.9	2	6606.643	1
87	6	max	0	10	.983	3	.023	3	1.062e-2	3	NC	5	NC	2
88		min	-.327	4	-1.279	2	-.007	10	-1.431e-2	2	248.487	2	7076.647	1
89	7	max	0	10	1.016	3	.026	3	1.141e-2	3	NC	5	NC	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.1	2	.022	4	1.428e-2	3	NC	4	NC	2
148			min	-.107	4	-.161	3	-.004	10	-1.404e-2	2	1257.828	2	6262.553	4
149		18	max	0	1	.151	2	.014	4	1.37e-2	3	NC	4	NC	1
150			min	-.107	4	-.174	3	-.004	2	-1.404e-2	2	2265.999	2	9291.936	4
151		19	max	0	1	.214	2	.009	3	1.312e-2	3	NC	1	NC	1
152			min	-.107	4	-.194	3	-.007	2	-1.403e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.013	2	.004	1	1.744e-3	5	NC	1	NC	1
154			min	-.012	3	-.019	3	-.491	4	-9.218e-5	1	5935.556	2	157.829	4
155		2	max	.008	2	.011	2	.004	1	1.756e-3	5	NC	1	NC	1
156			min	-.011	3	-.019	3	-.451	4	-8.775e-5	1	6837.933	2	171.689	4
157		3	max	.007	2	.01	2	.004	1	1.767e-3	5	NC	1	NC	1
158			min	-.011	3	-.018	3	-.412	4	-8.332e-5	1	8046.214	2	188.121	4
159		4	max	.007	2	.008	2	.003	1	1.779e-3	5	NC	1	NC	1
160			min	-.01	3	-.017	3	-.373	4	-7.889e-5	1	9722.158	2	207.798	4
161		5	max	.007	2	.006	2	.003	1	1.791e-3	5	NC	1	NC	1
162			min	-.009	3	-.017	3	-.335	4	-7.447e-5	1	NC	1	231.628	4
163		6	max	.006	2	.005	2	.003	1	1.802e-3	5	NC	1	NC	1
164			min	-.009	3	-.016	3	-.297	4	-7.004e-5	1	NC	1	260.87	4
165		7	max	.006	2	.003	2	.002	1	1.814e-3	5	NC	1	NC	1
166			min	-.008	3	-.015	3	-.261	4	-6.561e-5	1	NC	1	297.31	4
167		8	max	.005	2	.002	2	.002	1	1.826e-3	5	NC	1	NC	1
168			min	-.007	3	-.014	3	-.226	4	-6.118e-5	1	NC	1	343.549	4
169		9	max	.005	2	0	2	.002	1	1.837e-3	5	NC	1	NC	1
170			min	-.007	3	-.013	3	-.192	4	-5.675e-5	1	NC	1	403.494	4
171		10	max	.004	2	0	2	.001	1	1.849e-3	4	NC	1	NC	1
172			min	-.006	3	-.012	3	-.16	4	-5.232e-5	1	NC	1	483.251	4
173		11	max	.004	2	0	2	.001	1	1.862e-3	4	NC	1	NC	1
174			min	-.005	3	-.011	3	-.131	4	-4.79e-5	1	NC	1	592.786	4
175		12	max	.003	2	-.001	15	0	1	1.875e-3	4	NC	1	NC	1
176			min	-.005	3	-.01	3	-.103	4	-4.347e-5	1	NC	1	749.315	4
177		13	max	.003	2	-.001	15	0	1	1.888e-3	4	NC	1	NC	1
178			min	-.004	3	-.009	3	-.079	4	-3.904e-5	1	NC	1	984.727	4
179		14	max	.002	2	-.001	15	0	1	1.901e-3	4	NC	1	NC	1
180			min	-.003	3	-.008	3	-.057	4	-3.461e-5	1	NC	1	1363.743	4
181		15	max	.002	2	-.001	15	0	1	1.914e-3	4	NC	1	NC	1
182			min	-.003	3	-.006	3	-.038	4	-3.018e-5	1	NC	1	2035.112	4
183		16	max	.001	2	0	15	0	1	1.926e-3	4	NC	1	NC	1
184			min	-.002	3	-.005	3	-.023	4	-2.575e-5	1	NC	1	3408.361	4
185		17	max	0	2	0	15	0	1	1.939e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	3	-.011	4	-2.133e-5	1	NC	1	6992.651	4
187		18	max	0	2	0	15	0	1	1.952e-3	4	NC	1	NC	1
188			min	0	3	-.002	3	-.003	4	-1.69e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	1.965e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.247e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	2.577e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.251e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.011	4	5.983e-5	4	NC	1	NC	1
194			min	0	2	-.003	6	0	1	8.829e-7	10	NC	1	9671.26	4
195		3	max	.001	3	-.001	15	.02	4	5.447e-4	4	NC	1	NC	1
196			min	-.001	2	-.006	6	0	1	1.671e-6	10	NC	1	5029.826	4
197		4	max	.002	3	-.002	15	.029	4	1.03e-3	4	NC	1	NC	1
198			min	-.002	2	-.009	6	0	1	2.459e-6	10	NC	1	3487.894	4
199		5	max	.002	3	-.003	15	.037	4	1.515e-3	4	NC	1	NC	1
200			min	-.002	2	-.012	6	0	1	3.247e-6	10	8552.736	6	2719.648	4
201		6	max	.003	3	-.003	15	.045	4	1.999e-3	4	NC	2	NC	1
202			min	-.003	2	-.015	6	0	1	4.035e-6	10	6928.354	6	2259.59	4
203		7	max	.003	3	-.004	15	.052	4	2.484e-3	4	NC	5	NC	1





Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	10	5.648e-4	4	NC	1	NC	1
262		min	0	5	-0.001	3	-0.003	4	5.03e-6	10	NC	1	7337.412	4
263	18	max	0	1	0	2	0	10	5.648e-4	4	NC	1	NC	1
264		min	0	5	0	3	-0.001	4	5.03e-6	10	NC	1	NC	1
265	19	max	0	1	0	1	0	1	5.648e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	5.03e-6	10	NC	1	NC	1
267	M6	1	max	.024	2	.039	2	0	1.806e-3	4	NC	3	NC	1
268		min	-.035	3	-.055	3	-.496	4	0	1	2007.032	2	156.341	4
269	2	max	.022	2	.035	2	0	1	1.817e-3	4	NC	3	NC	1
270		min	-.034	3	-.052	3	-.456	4	0	1	2203.477	2	170.07	4
271	3	max	.021	2	.032	2	0	1	1.828e-3	4	NC	3	NC	1
272		min	-.032	3	-.049	3	-.416	4	0	1	2440.499	2	186.349	4
273	4	max	.02	2	.028	2	0	1	1.839e-3	4	NC	3	NC	1
274		min	-.03	3	-.046	3	-.376	4	0	1	2729.58	2	205.841	4
275	5	max	.018	2	.025	2	0	1	1.85e-3	4	NC	3	NC	1
276		min	-.028	3	-.043	3	-.338	4	0	1	3086.711	2	229.446	4
277	6	max	.017	2	.022	2	0	1	1.861e-3	4	NC	3	NC	1
278		min	-.026	3	-.04	3	-.3	4	0	1	3534.694	2	258.413	4
279	7	max	.016	2	.019	2	0	1	1.873e-3	4	NC	1	NC	1
280		min	-.024	3	-.037	3	-.263	4	0	1	4106.958	2	294.509	4
281	8	max	.014	2	.016	2	0	1	1.884e-3	4	NC	1	NC	1
282		min	-.022	3	-.034	3	-.228	4	0	1	4854.135	2	340.31	4
283	9	max	.013	2	.013	2	0	1	1.895e-3	4	NC	1	NC	1
284		min	-.02	3	-.031	3	-.194	4	0	1	5855.955	2	399.686	4
285	10	max	.012	2	.011	2	0	1	1.906e-3	4	NC	1	NC	1
286		min	-.018	3	-.028	3	-.162	4	0	1	7244.055	2	478.68	4
287	11	max	.011	2	.008	2	0	1	1.917e-3	4	NC	1	NC	1
288		min	-.016	3	-.025	3	-.132	4	0	1	9248.9	2	587.163	4
289	12	max	.009	2	.006	2	0	1	1.928e-3	4	NC	1	NC	1
290		min	-.014	3	-.021	3	-.104	4	0	1	NC	1	742.176	4
291	13	max	.008	2	.004	2	0	1	1.939e-3	4	NC	1	NC	1
292		min	-.012	3	-.018	3	-.079	4	0	1	NC	1	975.284	4
293	14	max	.007	2	.003	2	0	1	1.95e-3	4	NC	1	NC	1
294		min	-.01	3	-.015	3	-.057	4	0	1	NC	1	1350.534	4
295	15	max	.005	2	.002	2	0	1	1.961e-3	4	NC	1	NC	1
296		min	-.008	3	-.012	3	-.038	4	0	1	NC	1	2015.094	4
297	16	max	.004	2	0	2	0	1	1.972e-3	4	NC	1	NC	1
298		min	-.006	3	-.009	3	-.023	4	0	1	NC	1	3373.973	4
299	17	max	.003	2	0	2	0	1	1.983e-3	4	NC	1	NC	1
300		min	-.004	3	-.006	3	-.011	4	0	1	NC	1	6918.754	4
301	18	max	.001	2	0	2	0	1	1.995e-3	4	NC	1	NC	1
302		min	-.002	3	-.003	3	-.003	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	2.006e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-4.346e-4	4	NC	1	NC	1
307	2	max	.002	3	0	15	.011	4	3.697e-5	4	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	9472.094	4
309	3	max	.003	3	-.001	15	.021	4	5.085e-4	4	NC	1	NC	1
310		min	-.003	2	-.008	3	0	1	0	1	NC	1	4925.972	4
311	4	max	.005	3	-.002	15	.03	4	9.801e-4	4	NC	1	NC	1
312		min	-.004	2	-.011	3	0	1	0	1	NC	1	3416.772	4
313	5	max	.006	3	-.003	15	.038	4	1.452e-3	4	NC	1	NC	1
314		min	-.006	2	-.014	3	0	1	0	1	7950.39	3	2665.86	4
315	6	max	.008	3	-.004	15	.046	4	1.923e-3	4	NC	1	NC	1
316		min	-.007	2	-.017	3	0	1	0	1	6713.683	3	2217.207	4
317	7	max	.009	3	-.004	15	.053	4	2.395e-3	4	NC	2	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318			min	-0.009	2	-0.019	3	0	1	0	1	5911.417	4	1918.504	4
319		8	max	.011	3	-.005	15	.06	4	2.866e-3	4	NC	2	NC	1
320			min	-.01	2	-.021	3	0	1	0	1	5314.45	4	1704.191	4
321		9	max	.013	3	-.005	15	.066	4	3.338e-3	4	NC	5	NC	1
322			min	-.012	2	-.022	3	0	1	0	1	4962.341	4	1541.225	4
323		10	max	.014	3	-.005	15	.072	4	3.809e-3	4	NC	5	NC	1
324			min	-.013	2	-.023	3	0	1	0	1	4793.94	4	1411.025	4
325		11	max	.016	3	-.005	15	.078	4	4.281e-3	4	NC	5	NC	1
326			min	-.015	2	-.023	3	0	1	0	1	4784.499	4	1302.278	4
327		12	max	.017	3	-.005	15	.084	4	4.752e-3	4	NC	5	NC	1
328			min	-.016	2	-.022	3	0	1	0	1	4936.187	4	1207.701	4
329		13	max	.019	3	-.005	15	.091	4	5.224e-3	4	NC	5	NC	1
330			min	-.018	2	-.021	3	0	1	0	1	5280.148	4	1122.446	4
331		14	max	.021	3	-.004	15	.097	4	5.696e-3	4	NC	2	NC	1
332			min	-.019	2	-.02	3	0	1	0	1	5892.557	4	1043.274	4
333		15	max	.022	3	-.004	15	.105	4	6.167e-3	4	NC	1	NC	1
334			min	-.021	2	-.018	3	0	1	0	1	6941.947	4	968.083	4
335		16	max	.024	3	-.003	15	.114	4	6.639e-3	4	NC	1	NC	1
336			min	-.022	2	-.015	3	0	1	0	1	8836.252	4	895.629	4
337		17	max	.025	3	-.002	15	.123	4	7.11e-3	4	NC	1	NC	1
338			min	-.024	2	-.013	3	0	1	0	1	NC	1	825.321	4
339		18	max	.027	3	-.001	15	.134	4	7.582e-3	4	NC	1	NC	1
340			min	-.025	2	-.01	3	0	1	0	1	NC	1	757.052	4
341		19	max	.028	3	0	10	.147	4	8.053e-3	4	NC	1	NC	1
342			min	-.027	2	-.007	3	0	1	0	1	NC	1	691.047	4
343	M8	1	max	.006	2	.026	2	0	1	4.031e-4	4	NC	1	NC	1
344			min	0	3	-.029	3	-.147	4	0	1	NC	1	168.611	4
345		2	max	.005	2	.025	2	0	1	4.031e-4	4	NC	1	NC	1
346			min	0	3	-.028	3	-.135	4	0	1	NC	1	183.169	4
347		3	max	.005	2	.023	2	0	1	4.031e-4	4	NC	1	NC	1
348			min	0	3	-.026	3	-.124	4	0	1	NC	1	200.504	4
349		4	max	.005	2	.022	2	0	1	4.031e-4	4	NC	1	NC	1
350			min	0	3	-.024	3	-.112	4	0	1	NC	1	221.336	4
351		5	max	.004	2	.02	2	0	1	4.031e-4	4	NC	1	NC	1
352			min	0	3	-.023	3	-.101	4	0	1	NC	1	246.643	4
353		6	max	.004	2	.019	2	0	1	4.031e-4	4	NC	1	NC	1
354			min	0	3	-.021	3	-.089	4	0	1	NC	1	277.781	4
355		7	max	.004	2	.018	2	0	1	4.031e-4	4	NC	1	NC	1
356			min	0	3	-.019	3	-.078	4	0	1	NC	1	316.675	4
357		8	max	.003	2	.016	2	0	1	4.031e-4	4	NC	1	NC	1
358			min	0	3	-.018	3	-.068	4	0	1	NC	1	366.133	4
359		9	max	.003	2	.015	2	0	1	4.031e-4	4	NC	1	NC	1
360			min	0	3	-.016	3	-.058	4	0	1	NC	1	430.379	4
361		10	max	.003	2	.013	2	0	1	4.031e-4	4	NC	1	NC	1
362			min	0	3	-.015	3	-.048	4	0	1	NC	1	516.02	4
363		11	max	.003	2	.012	2	0	1	4.031e-4	4	NC	1	NC	1
364			min	0	3	-.013	3	-.039	4	0	1	NC	1	633.857	4
365		12	max	.002	2	.01	2	0	1	4.031e-4	4	NC	1	NC	1
366			min	0	3	-.011	3	-.031	4	0	1	NC	1	802.575	4
367		13	max	.002	2	.009	2	0	1	4.031e-4	4	NC	1	NC	1
368			min	0	3	-.01	3	-.023	4	0	1	NC	1	1056.842	4
369		14	max	.002	2	.007	2	0	1	4.031e-4	4	NC	1	NC	1
370			min	0	3	-.008	3	-.017	4	0	1	NC	1	1467.171	4
371		15	max	.001	2	.006	2	0	1	4.031e-4	4	NC	1	NC	1
372			min	0	3	-.006	3	-.011	4	0	1	NC	1	2196.055	4
373		16	max	0	2	.004	2	0	1	4.031e-4	4	NC	1	NC	1
374			min	0	3	-.005	3	-.007	4	0	1	NC	1	3692.491	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.003	2	0	1	4.031e-4	4	NC	1	NC	1
376			min	0	3	-.003	3	-.003	4	0	1	NC	1	7620.265	4
377		18	max	0	2	.001	2	0	1	4.031e-4	4	NC	1	NC	1
378			min	0	3	-.002	3	0	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	4.031e-4	4	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	2	.013	2	0	10	1.794e-3	4	NC	1	NC	1
382			min	-.012	3	-.019	3	-.494	4	3.47e-6	10	5935.556	2	156.931	4
383		2	max	.008	2	.011	2	0	10	1.804e-3	4	NC	1	NC	1
384			min	-.011	3	-.019	3	-.454	4	3.304e-6	10	6837.933	2	170.714	4
385		3	max	.007	2	.01	2	0	10	1.814e-3	4	NC	1	NC	1
386			min	-.011	3	-.018	3	-.414	4	3.138e-6	10	8046.214	2	187.055	4
387		4	max	.007	2	.008	2	0	10	1.824e-3	4	NC	1	NC	1
388			min	-.01	3	-.017	3	-.375	4	2.972e-6	10	9722.158	2	206.623	4
389		5	max	.007	2	.006	2	0	10	1.834e-3	4	NC	1	NC	1
390			min	-.009	3	-.017	3	-.336	4	2.806e-6	10	NC	1	230.321	4
391		6	max	.006	2	.005	2	0	10	1.844e-3	4	NC	1	NC	1
392			min	-.009	3	-.016	3	-.299	4	2.639e-6	10	NC	1	259.402	4
393		7	max	.006	2	.003	2	0	10	1.854e-3	4	NC	1	NC	1
394			min	-.008	3	-.015	3	-.262	4	2.473e-6	10	NC	1	295.642	4
395		8	max	.005	2	.002	2	0	10	1.864e-3	4	NC	1	NC	1
396			min	-.007	3	-.014	3	-.227	4	2.307e-6	10	NC	1	341.627	4
397		9	max	.005	2	0	2	0	10	1.874e-3	4	NC	1	NC	1
398			min	-.007	3	-.013	3	-.193	4	2.141e-6	10	NC	1	401.245	4
399		10	max	.004	2	0	2	0	10	1.884e-3	4	NC	1	NC	1
400			min	-.006	3	-.012	3	-.161	4	1.975e-6	10	NC	1	480.566	4
401		11	max	.004	2	0	2	0	10	1.893e-3	4	NC	1	NC	1
402			min	-.005	3	-.011	3	-.131	4	1.808e-6	10	NC	1	589.507	4
403		12	max	.003	2	-.002	2	0	10	1.903e-3	4	NC	1	NC	1
404			min	-.005	3	-.01	3	-.104	4	1.642e-6	10	NC	1	745.188	4
405		13	max	.003	2	-.002	2	0	10	1.913e-3	4	NC	1	NC	1
406			min	-.004	3	-.009	3	-.079	4	1.476e-6	10	NC	1	979.331	4
407		14	max	.002	2	-.002	15	0	10	1.923e-3	4	NC	1	NC	1
408			min	-.003	3	-.008	3	-.057	4	1.31e-6	10	NC	1	1356.314	4
409		15	max	.002	2	-.002	15	0	10	1.933e-3	4	NC	1	NC	1
410			min	-.003	3	-.006	3	-.038	4	1.144e-6	10	NC	1	2024.104	4
411		16	max	.001	2	-.001	15	0	10	1.943e-3	4	NC	1	NC	1
412			min	-.002	3	-.005	4	-.023	4	9.774e-7	10	NC	1	3390.103	4
413		17	max	0	2	0	15	0	10	1.953e-3	4	NC	1	NC	1
414			min	-.001	3	-.004	4	-.011	4	8.112e-7	10	NC	1	6955.748	4
415		18	max	0	2	0	15	0	10	1.963e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.003	4	6.45e-7	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	1.973e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	4.788e-7	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-9.487e-8	10	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.267e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.011	4	5.261e-5	5	NC	1	NC	1
422			min	0	2	-.003	4	0	10	-1.374e-5	1	NC	1	9632.858	4
423		3	max	.001	3	-.002	15	.02	4	5.287e-4	4	NC	1	NC	1
424			min	-.001	2	-.006	4	0	10	-2.49e-5	1	NC	1	5010.884	4
425		4	max	.002	3	-.002	15	.029	4	1.006e-3	4	NC	1	NC	1
426			min	-.002	2	-.009	4	0	10	-3.607e-5	1	NC	1	3475.883	4
427		5	max	.002	3	-.003	15	.037	4	1.484e-3	4	NC	1	NC	1
428			min	-.002	2	-.013	4	0	10	-4.723e-5	1	8240.689	4	2711.517	4
429		6	max	.003	3	-.004	15	.045	4	1.962e-3	4	NC	2	NC	1
430			min	-.003	2	-.016	4	0	10	-5.839e-5	1	6697.348	4	2254.191	4
431		7	max	.003	3	-.004	15	.052	4	2.44e-3	4	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.003	2	-.018	4	0	10	-6.956e-5	1	5767.012	4	1949.057	4
433		8	max	.004	3	-.005	15	.059	4	2.917e-3	4	NC	5	NC	1
434			min	-.004	2	-.02	4	0	10	-8.072e-5	1	5193.586	4	1729.456	4
435		9	max	.005	3	-.005	15	.065	4	3.395e-3	4	NC	5	NC	1
436			min	-.004	2	-.022	4	0	1	-9.189e-5	1	4856.479	4	1561.797	4
437		10	max	.005	3	-.006	15	.071	4	3.873e-3	4	NC	5	NC	1
438			min	-.005	2	-.022	4	0	1	-1.03e-4	1	4697.362	4	1427.209	4
439		11	max	.006	3	-.006	15	.077	4	4.351e-3	4	NC	5	NC	1
440			min	-.005	2	-.022	4	0	1	-1.142e-4	1	4692.913	4	1314.234	4
441		12	max	.006	3	-.005	15	.084	4	4.828e-3	4	NC	5	NC	1
442			min	-.006	2	-.022	4	0	1	-1.254e-4	1	4845.889	4	1215.534	4
443		13	max	.007	3	-.005	15	.09	4	5.306e-3	4	NC	5	NC	1
444			min	-.006	2	-.021	4	0	1	-1.365e-4	1	5187.333	4	1126.277	4
445		14	max	.007	3	-.005	15	.097	4	5.784e-3	4	NC	5	NC	1
446			min	-.007	2	-.019	4	-.001	1	-1.477e-4	1	5792.49	4	1043.286	4
447		15	max	.008	3	-.004	15	.105	4	6.261e-3	4	NC	3	NC	1
448			min	-.007	2	-.016	4	-.002	1	-1.589e-4	1	6827.44	4	964.556	4
449		16	max	.009	3	-.003	15	.114	4	6.739e-3	4	NC	1	NC	1
450			min	-.008	2	-.013	4	-.002	1	-1.7e-4	1	8693.889	4	888.947	4
451		17	max	.009	3	-.003	15	.125	4	7.217e-3	4	NC	1	NC	1
452			min	-.008	2	-.009	4	-.003	1	-1.812e-4	1	NC	1	815.964	4
453		18	max	.01	3	-.002	15	.136	4	7.695e-3	4	NC	1	NC	1
454			min	-.009	2	-.005	4	-.003	1	-1.924e-4	1	NC	1	745.573	4
455		19	max	.01	3	0	10	.15	4	8.172e-3	4	NC	1	NC	1
456			min	-.009	2	-.002	3	-.004	1	-2.035e-4	1	NC	1	678.032	4
457	M12	1	max	.002	1	.009	2	.004	1	5.239e-4	5	NC	1	NC	2
458			min	0	3	-.011	3	-.15	4	-8.087e-5	1	NC	1	165.435	4
459		2	max	.002	1	.008	2	.004	1	5.239e-4	5	NC	1	NC	2
460			min	0	3	-.01	3	-.138	4	-8.087e-5	1	NC	1	179.702	4
461		3	max	.002	1	.008	2	.003	1	5.239e-4	5	NC	1	NC	2
462			min	0	3	-.009	3	-.126	4	-8.087e-5	1	NC	1	196.691	4
463		4	max	.002	1	.007	2	.003	1	5.239e-4	5	NC	1	NC	2
464			min	0	3	-.009	3	-.114	4	-8.087e-5	1	NC	1	217.108	4
465		5	max	.002	1	.007	2	.003	1	5.239e-4	5	NC	1	NC	2
466			min	0	3	-.008	3	-.103	4	-8.087e-5	1	NC	1	241.912	4
467		6	max	.001	1	.006	2	.003	1	5.239e-4	5	NC	1	NC	2
468			min	0	3	-.008	3	-.091	4	-8.087e-5	1	NC	1	272.431	4
469		7	max	.001	1	.006	2	.002	1	5.239e-4	5	NC	1	NC	1
470			min	0	3	-.007	3	-.08	4	-8.087e-5	1	NC	1	310.554	4
471		8	max	.001	1	.005	2	.002	1	5.239e-4	5	NC	1	NC	1
472			min	0	3	-.006	3	-.069	4	-8.087e-5	1	NC	1	359.031	4
473		9	max	.001	1	.005	2	.002	1	5.239e-4	5	NC	1	NC	1
474			min	0	3	-.006	3	-.059	4	-8.087e-5	1	NC	1	422.003	4
475		10	max	.001	1	.004	2	.001	1	5.239e-4	5	NC	1	NC	1
476			min	0	3	-.005	3	-.049	4	-8.087e-5	1	NC	1	505.945	4
477		11	max	0	1	.004	2	.001	1	5.239e-4	5	NC	1	NC	1
478			min	0	3	-.005	3	-.04	4	-8.087e-5	1	NC	1	621.444	4
479		12	max	0	1	.003	2	0	1	5.239e-4	5	NC	1	NC	1
480			min	0	3	-.004	3	-.032	4	-8.087e-5	1	NC	1	786.811	4
481		13	max	0	1	.003	2	0	1	5.239e-4	5	NC	1	NC	1
482			min	0	3	-.004	3	-.024	4	-8.087e-5	1	NC	1	1036.024	4
483		14	max	0	1	.002	2	0	1	5.239e-4	5	NC	1	NC	1
484			min	0	3	-.003	3	-.017	4	-8.087e-5	1	NC	1	1438.19	4
485		15	max	0	1	.002	2	0	1	5.239e-4	5	NC	1	NC	1
486			min	0	3	-.002	3	-.012	4	-8.087e-5	1	NC	1	2152.553	4
487		16	max	0	1	.001	2	0	1	5.239e-4	5	NC	1	NC	1
488			min	0	3	-.002	3	-.007	4	-8.087e-5	1	NC	1	3619.13	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	5.239e-4	5	NC	1	NC	1
490			min	0	3	-.001	3	-.003	4	-8.087e-5	1	NC	1	7468.349	4
491		18	max	0	1	0	2	0	1	5.239e-4	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-8.087e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	5.239e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-8.087e-5	1	NC	1	NC	1
495	M1	1	max	.013	3	.241	2	.52	4	4.358e-3	2	NC	1	NC	1
496			min	-.008	2	-.086	3	0	10	-1.278e-2	3	NC	1	NC	1
497		2	max	.013	3	.116	2	.506	4	4.887e-3	4	NC	5	NC	1
498			min	-.008	2	-.04	3	-.003	1	-6.351e-3	3	1088.96	2	NC	1
499		3	max	.013	3	.02	3	.491	4	9.093e-3	4	NC	5	NC	1
500			min	-.009	2	-.015	2	-.004	1	-9.185e-5	3	529.431	2	7709.589	5
501		4	max	.013	3	.106	3	.474	4	7.76e-3	4	NC	5	NC	1
502			min	-.008	2	-.159	2	-.004	1	-3.463e-3	3	338.949	2	5680.64	5
503		5	max	.012	3	.209	3	.457	4	6.622e-3	2	NC	15	NC	1
504			min	-.008	2	-.307	2	-.003	1	-6.835e-3	3	247.512	2	4653.941	5
505		6	max	.012	3	.317	3	.439	4	9.921e-3	2	9832.42	15	NC	1
506			min	-.008	2	-.448	2	-.001	1	-1.021e-2	3	196.72	2	4009.552	5
507		7	max	.012	3	.418	3	.42	4	1.322e-2	2	8340.682	15	NC	1
508			min	-.008	2	-.573	2	0	3	-1.358e-2	3	166.535	2	3519.085	4
509		8	max	.012	3	.502	3	.402	4	1.652e-2	2	7454.651	15	NC	1
510			min	-.008	2	-.672	2	0	10	-1.695e-2	3	148.598	2	3113.172	4
511		9	max	.011	3	.556	3	.384	4	1.839e-2	2	6989.265	15	NC	1
512			min	-.008	2	-.734	2	0	1	-1.763e-2	3	139.225	2	2801.432	4
513		10	max	.011	3	.576	3	.364	4	1.932e-2	2	6846.442	15	NC	1
514			min	-.007	2	-.755	2	0	10	-1.652e-2	3	136.494	2	2671.428	4
515		11	max	.011	3	.562	3	.341	4	2.024e-2	2	6988.623	15	NC	1
516			min	-.007	2	-.733	2	0	10	-1.541e-2	3	139.769	2	2660.553	4
517		12	max	.01	3	.516	3	.316	4	1.926e-2	2	7453.129	15	NC	1
518			min	-.007	2	-.668	2	0	1	-1.364e-2	3	150.152	2	2755.238	4
519		13	max	.01	3	.441	3	.287	4	1.544e-2	2	8337.759	15	NC	1
520			min	-.007	2	-.565	2	0	1	-1.092e-2	3	170.121	2	3187.273	4
521		14	max	.01	3	.344	3	.253	4	1.161e-2	2	9827.167	15	NC	1
522			min	-.007	2	-.435	2	0	10	-8.187e-3	3	204.096	2	4223.558	4
523		15	max	.01	3	.235	3	.218	4	7.792e-3	2	NC	15	NC	1
524			min	-.007	2	-.291	2	0	10	-5.458e-3	3	262.183	2	6740.514	4
525		16	max	.009	3	.12	3	.184	4	6.706e-3	4	NC	5	NC	1
526			min	-.007	2	-.145	2	0	10	-2.729e-3	3	368.78	2	NC	1
527		17	max	.009	3	.007	3	.153	4	7.801e-3	4	NC	5	NC	1
528			min	-.007	2	-.008	2	0	10	-4.775e-8	3	594.396	2	NC	1
529		18	max	.009	3	.108	2	.127	4	4.22e-3	4	NC	5	NC	1
530			min	-.007	2	-.096	3	0	10	-1.293e-3	3	1250.846	2	NC	1
531		19	max	.009	3	.214	2	.107	4	8.284e-3	2	NC	1	NC	1
532			min	-.007	2	-.194	3	0	1	-2.658e-3	3	NC	1	NC	1
533	M5	1	max	.036	3	.309	2	.52	4	0	1	NC	1	NC	1
534			min	-.026	2	.006	15	0	1	-1.765e-5	4	NC	1	NC	1
535		2	max	.036	3	.147	2	.51	4	4.646e-3	4	NC	5	NC	1
536			min	-.026	2	.003	15	0	1	0	1	849.164	2	NC	1
537		3	max	.036	3	.056	3	.495	4	9.197e-3	4	NC	5	NC	1
538			min	-.026	2	-.041	2	0	1	0	1	392.685	2	6353.888	4
539		4	max	.035	3	.181	3	.478	4	7.492e-3	4	NC	15	NC	1
540			min	-.025	2	-.274	2	0	1	0	1	235.244	2	4999.158	4
541		5	max	.034	3	.364	3	.459	4	5.788e-3	4	8967.111	15	NC	1
542			min	-.025	2	-.532	2	0	1	0	1	162.648	2	4358.671	4
543		6	max	.033	3	.575	3	.439	4	4.083e-3	4	6837.911	15	NC	1
544			min	-.024	2	-.793	2	0	1	0	1	124.071	2	3948.425	4
545		7	max	.033	3	.783	3	.42	4	2.379e-3	4	5620.875	15	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.024	2	-1.031	2	0	1	0	1	101.962	2	3578.654	4
547		8	max	.032	3	.959	3	.402	4	6.745e-4	4	4918.787	15	NC	1
548			min	-.023	2	-1.224	2	0	1	0	1	89.182	2	3164.285	4
549		9	max	.031	3	1.073	3	.385	4	0	1	4560.329	15	NC	1
550			min	-.023	2	-1.347	2	0	1	-1.14e-5	5	82.647	2	2789.518	4
551		10	max	.03	3	1.113	3	.363	4	0	1	4452.566	15	NC	1
552			min	-.022	2	-1.389	2	0	1	-1.101e-5	5	80.746	2	2698.135	4
553		11	max	.03	3	1.084	3	.34	4	0	1	4560.962	15	NC	1
554			min	-.022	2	-1.348	2	0	1	-1.062e-5	5	83.012	2	2705.585	4
555		12	max	.029	3	.988	3	.317	4	5.466e-4	4	4920.246	15	NC	1
556			min	-.022	2	-1.22	2	0	1	0	1	90.433	2	2699.727	4
557		13	max	.028	3	.833	3	.288	4	1.929e-3	4	5623.706	15	NC	1
558			min	-.021	2	-1.014	2	0	1	0	1	105.335	2	3089.673	4
559		14	max	.027	3	.639	3	.253	4	3.311e-3	4	6843.23	15	NC	1
560			min	-.021	2	-.761	2	0	1	0	1	131.984	2	4248.044	4
561		15	max	.027	3	.426	3	.216	4	4.693e-3	4	8977.383	15	NC	1
562			min	-.021	2	-.491	2	0	1	0	1	180.746	2	7720.83	4
563		16	max	.026	3	.212	3	.18	4	6.075e-3	4	NC	15	NC	1
564			min	-.02	2	-.235	2	0	1	0	1	278.801	2	NC	1
565		17	max	.025	3	.018	3	.148	4	7.457e-3	4	NC	5	NC	1
566			min	-.02	2	-.021	2	0	1	0	1	505.419	3	NC	1
567		18	max	.025	3	.125	2	.123	4	3.769e-3	4	NC	5	NC	1
568			min	-.02	2	-.141	3	0	1	0	1	1093.732	3	NC	1
569		19	max	.025	3	.234	2	.107	4	0	1	NC	1	NC	1
570			min	-.02	2	-.28	3	0	1	-1.036e-5	4	NC	1	NC	1
571	M9	1	max	.013	3	.241	2	.52	4	1.278e-2	3	NC	1	NC	1
572			min	-.008	2	-.086	3	0	1	-4.358e-3	2	NC	1	NC	1
573		2	max	.013	3	.116	2	.508	4	6.351e-3	3	NC	5	NC	1
574			min	-.008	2	-.04	3	0	10	-2.143e-3	2	1088.96	2	NC	1
575		3	max	.013	3	.02	3	.493	4	9.154e-3	4	NC	5	NC	1
576			min	-.009	2	-.015	2	0	10	-2.919e-5	10	529.431	2	6941.515	4
577		4	max	.013	3	.106	3	.477	4	7.279e-3	5	NC	5	NC	1
578			min	-.008	2	-.159	2	0	10	-3.323e-3	2	338.949	2	5278.643	4
579		5	max	.012	3	.209	3	.458	4	6.835e-3	3	NC	15	NC	1
580			min	-.008	2	-.307	2	0	10	-6.622e-3	2	247.512	2	4455.197	4
581		6	max	.012	3	.317	3	.439	4	1.021e-2	3	9780.848	15	NC	1
582			min	-.008	2	-.448	2	0	10	-9.921e-3	2	196.72	2	3933.625	4
583		7	max	.012	3	.418	3	.42	4	1.358e-2	3	8297.894	15	NC	1
584			min	-.008	2	-.573	2	0	1	-1.322e-2	2	166.535	2	3520.743	4
585		8	max	.012	3	.502	3	.402	4	1.695e-2	3	7416.978	15	NC	1
586			min	-.008	2	-.672	2	0	1	-1.652e-2	2	148.598	2	3130.83	4
587		9	max	.011	3	.556	3	.384	4	1.763e-2	3	6954.197	15	NC	1
588			min	-.008	2	-.734	2	0	10	-1.839e-2	2	139.225	2	2794.103	4
589		10	max	.011	3	.576	3	.364	4	1.652e-2	3	6812.069	15	NC	1
590			min	-.007	2	-.755	2	0	1	-1.932e-2	2	136.494	2	2672.198	4
591		11	max	.011	3	.562	3	.341	4	1.541e-2	3	6953.355	15	NC	1
592			min	-.007	2	-.733	2	0	1	-2.024e-2	2	139.769	2	2668.263	4
593		12	max	.01	3	.516	3	.316	4	1.364e-2	3	7415.249	15	NC	1
594			min	-.007	2	-.668	2	0	10	-1.926e-2	2	150.152	2	2741.159	4
595		13	max	.01	3	.441	3	.287	4	1.092e-2	3	8294.99	15	NC	1
596			min	-.007	2	-.565	2	0	10	-1.544e-2	2	170.121	2	3182.059	4
597		14	max	.01	3	.344	3	.253	4	8.187e-3	3	9776.13	15	NC	1
598			min	-.007	2	-.435	2	-.001	1	-1.161e-2	2	204.096	2	4299.81	5
599		15	max	.01	3	.235	3	.217	4	5.458e-3	3	NC	15	NC	1
600			min	-.007	2	-.291	2	-.003	1	-7.792e-3	2	262.183	2	7133.317	5
601		16	max	.009	3	.12	3	.182	4	6.124e-3	5	NC	5	NC	1
602			min	-.007	2	-.145	2	-.004	1	-3.97e-3	2	368.78	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.009	3	.007	3	.15	4	7.635e-3	4	NC	5	NC	1
604		min	-.007	2	-.008	2	-.004	1	-2.891e-4	1	594.396	2	NC	1
605	18	max	.009	3	.108	2	.125	4	3.786e-3	5	NC	5	NC	1
606		min	-.007	2	-.096	3	-.003	1	-4.146e-3	2	1250.846	2	NC	1
607	19	max	.009	3	.214	2	.107	4	2.658e-3	3	NC	1	NC	1
608		min	-.007	2	-.194	3	0	10	-8.284e-3	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

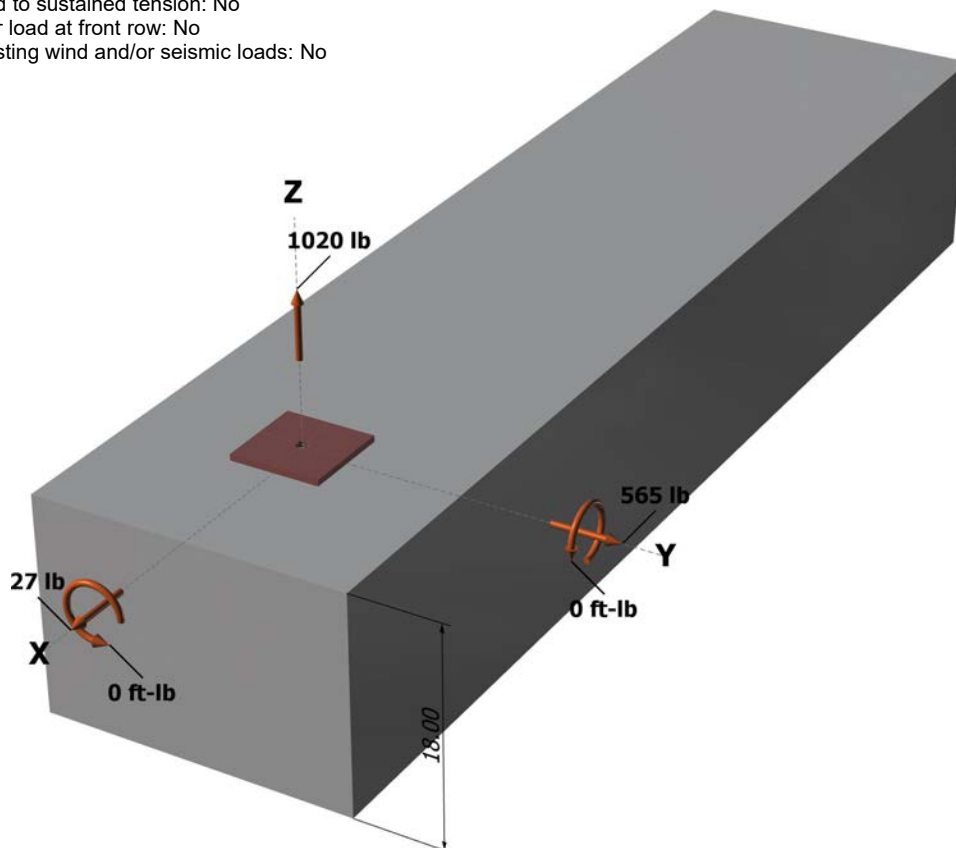
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbv} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbv} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

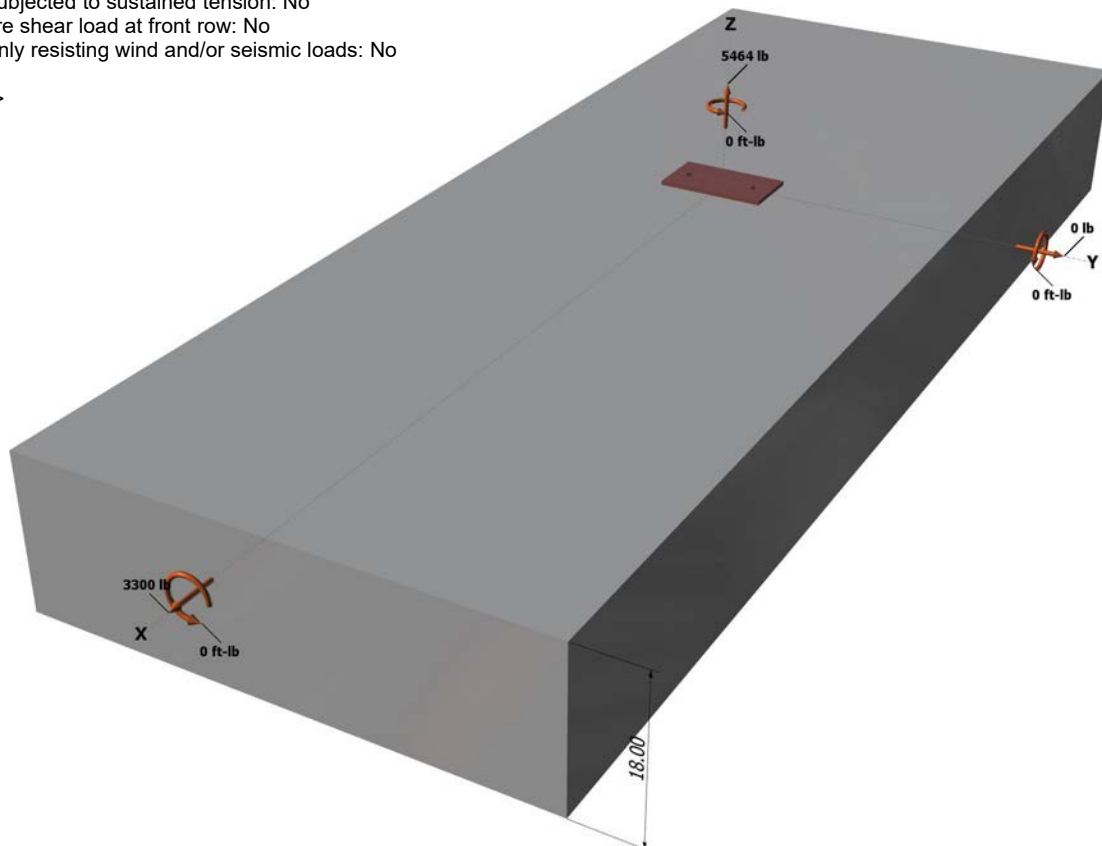
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



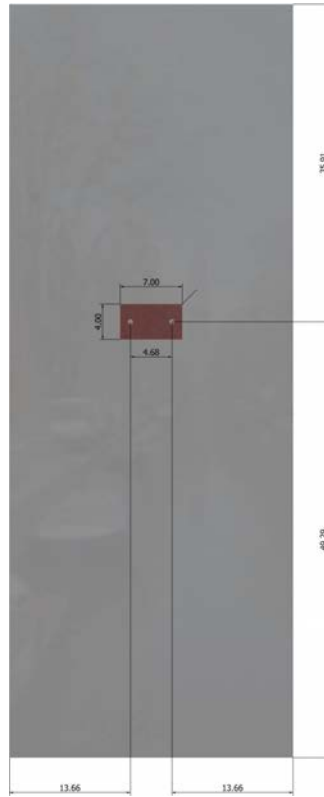
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.6025.0

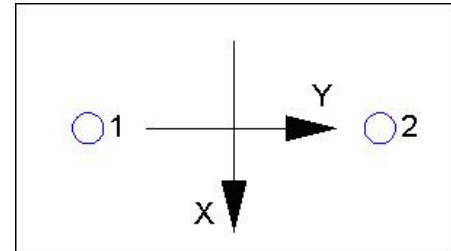
Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 5464
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.