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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

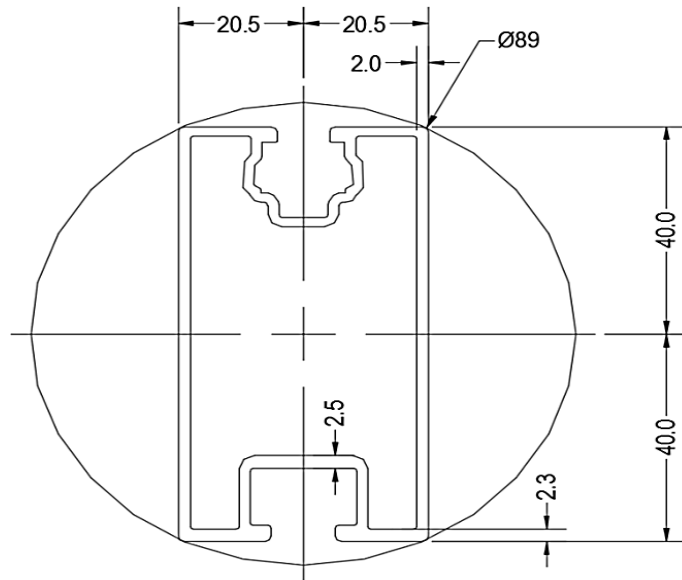
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	75 in
$\Phi F_{ty}$ STRONG-AXIS =	29.31 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	0.789 k-ft
$M_z$ =	0.109 k-ft
$M_{y \text{ allowable}}$ =	1.821 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>56%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

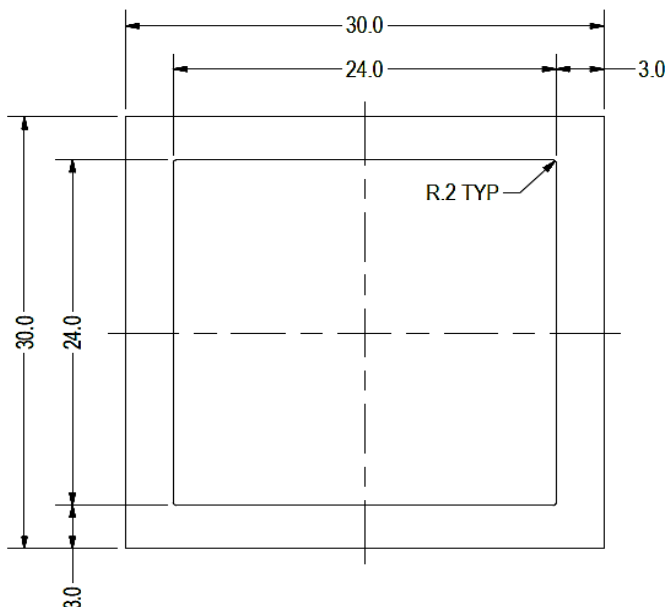
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.50 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.526 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.176 k
$M_{y \text{ allowable}}$ =	1.448 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>38%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

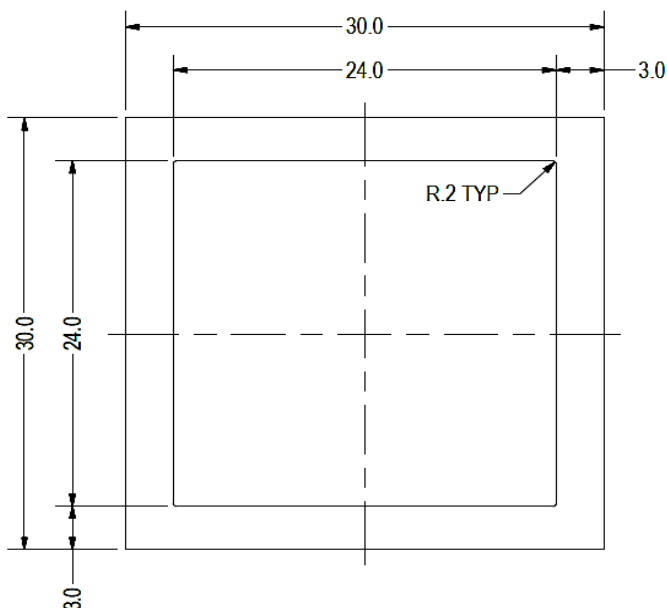
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.426 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>12%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

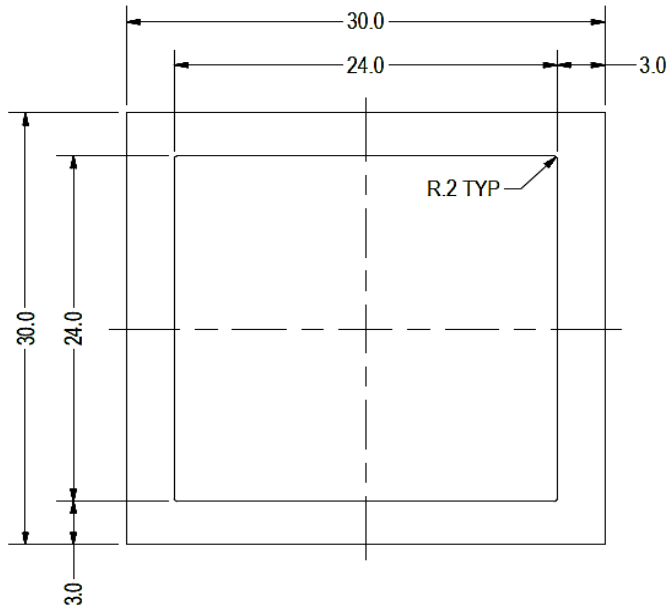
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.180 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>5%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

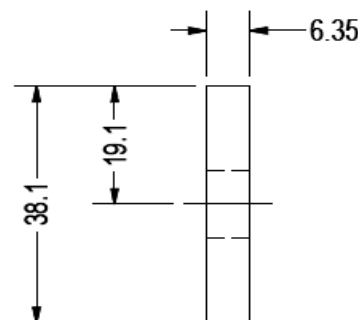
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.118 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>14%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.004 k-ft
$P_n$ =	0.040 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>9%</b>



A cross brace kit is required every 21 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

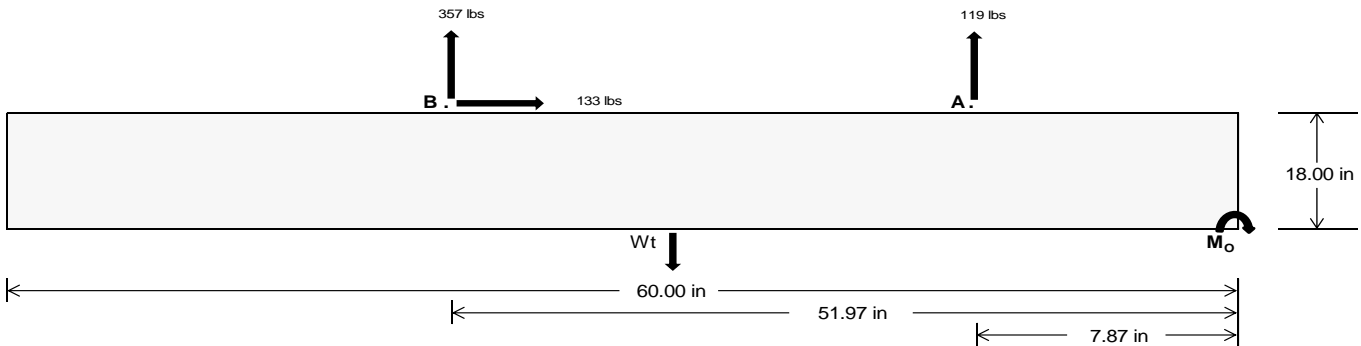
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>521.85</u>	<u>1553.96</u>	k
Compressive Load =	<u>1854.07</u>	<u>1348.97</u>	k
Lateral Load =	<u>2.25</u>	<u>578.22</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21907.9$  in-lbs  
Resisting Force Required = 730.26 lbs  
S.F. = 1.67  
Weight Required = 1217.11 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 133.38 lbs  
Friction = 0.4  
Weight Required = 333.45 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 133.38 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	653 lbs	653 lbs	653 lbs	653 lbs	592 lbs	592 lbs	592 lbs	592 lbs	889 lbs	889 lbs	889 lbs	889 lbs	-237 lbs	-237 lbs	-237 lbs	-237 lbs
$F_B$	476 lbs	476 lbs	476 lbs	476 lbs	431 lbs	431 lbs	431 lbs	431 lbs	646 lbs	646 lbs	646 lbs	646 lbs	-715 lbs	-715 lbs	-715 lbs	-715 lbs
$F_V$	41 lbs	41 lbs	41 lbs	41 lbs	235 lbs	235 lbs	235 lbs	235 lbs	205 lbs	205 lbs	205 lbs	205 lbs	-267 lbs	-267 lbs	-267 lbs	-267 lbs
$P_{total}$	3123 lbs	3213 lbs	3304 lbs	3395 lbs	3017 lbs	3107 lbs	3198 lbs	3289 lbs	3529 lbs	3619 lbs	3710 lbs	3801 lbs	244 lbs	298 lbs	353 lbs	407 lbs
$M$	393 lbs-ft	393 lbs-ft	393 lbs-ft	393 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	762 lbs-ft	762 lbs-ft	762 lbs-ft	762 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft
$e$	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	1.93 ft	1.58 ft	1.33 ft	1.16 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	289.2 psf	286.1 psf	283.2 psf	280.6 psf	243.3 psf	242.1 psf	241.1 psf	240.2 psf	285.2 psf	282.3 psf	279.6 psf	277.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	392.1 psf	384.5 psf	377.6 psf	371.2 psf	414.9 psf	406.4 psf	398.5 psf	391.3 psf	484.7 psf	473.1 psf	462.4 psf	452.6 psf	155.2 psf	112.4 psf	100.8 psf	96.9 psf

Maximum Bearing Pressure = 485 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

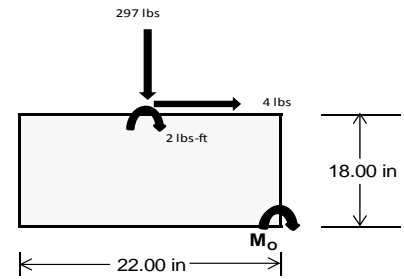
## Overturning Check

$M_o = 264.0$  ft-lbs  
 Resisting Force Required = 287.95 lbs  
 S.F. = 1.67  
 Weight Required = 479.92 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	72 lbs	196 lbs	68 lbs	297 lbs	915 lbs	293 lbs	21 lbs	57 lbs	20 lbs
$F_v$	1 lbs	1 lbs	0 lbs	4 lbs	4 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2540 lbs	2664 lbs	2536 lbs	2647 lbs	3264 lbs	2643 lbs	743 lbs	779 lbs	742 lbs
$M$	1 lbs-ft	1 lbs-ft	0 lbs-ft	9 lbs-ft	6 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	276.7 sqft	290.3 sqft	276.6 sqft	285.7 sqft	353.9 sqft	288.1 sqft	80.9 sqft	84.9 sqft	80.9 sqft
$f_{max}$	277.5 psf	291.0 psf	276.7 psf	291.8 psf	358.3 psf	288.5 psf	81.1 psf	85.1 psf	80.9 psf



Maximum Bearing Pressure = 358 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



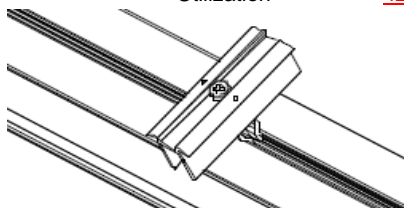
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

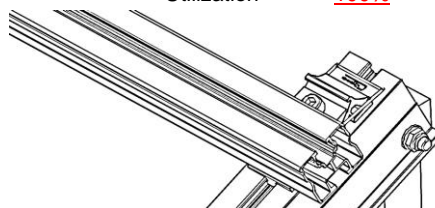
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.505 k
Allowable Uplift =	1.214 k
Utilization =	<u>42%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.113 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



### 6.2 Bolted Connections

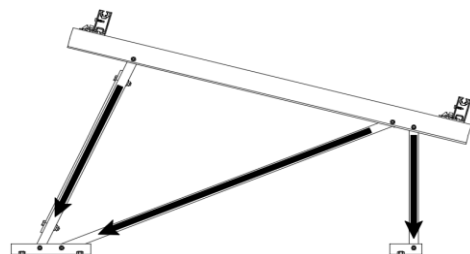
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.426 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

#### Diagonal Strut

Maximum Axial Load =	0.180 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.194 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

#### Bracing

Maximum Axial Load =	0.040 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.016 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.427$$

$$156.423$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.3 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.427$$

$$169.977$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.3 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.821 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.15 \\ &22.8869 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.5 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.15 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.5 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.448 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	107.933	2	297.861	1	.034	9	0	1	0	1	0	1
2		min	-141.248	3	-367.141	3	-.131	3	0	3	0	1	0	1
3	N7	max	0	15	476.383	1	-.025	15	0	15	0	1	0	1
4		min	-.145	2	-115.864	3	-.695	1	-.001	1	0	1	0	1
5	N15	max	0	15	1426.208	1	.422	1	0	1	0	1	0	1
6		min	-1.368	1	-401.421	3	-.304	3	0	3	0	1	0	1
7	N16	max	408.01	2	1037.668	1	0	10	0	1	0	1	0	1
8		min	-444.785	3	-1195.357	3	-35.847	3	0	3	0	1	0	1
9	N23	max	0	15	476.348	1	1.732	1	.003	1	0	1	0	1
10		min	-.145	2	-115.5	3	.058	15	0	15	0	1	0	1
11	N24	max	108.133	2	302.14	1	36.176	3	.001	1	0	1	0	1
12		min	-141.373	3	-364.994	3	.007	10	0	3	0	1	0	1
13	Totals:	max	622.437	2	4016.609	1	0	2						
14		min	-727.773	3	-2560.278	3	0	14						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	345.114	1	.664	4	.507	1	0	15	0	3	0	1
2			min	-369.566	3	.157	15	-.084	3	0	1	0	2	0	1
3		2	max	345.211	1	.626	4	.507	1	0	15	0	1	0	15
4			min	-369.494	3	.149	15	-.084	3	0	1	0	10	0	4
5		3	max	345.307	1	.588	4	.507	1	0	15	0	1	0	15
6			min	-369.421	3	.14	15	-.084	3	0	1	0	3	0	4
7		4	max	345.403	1	.551	4	.507	1	0	15	0	1	0	15
8			min	-369.349	3	.131	15	-.084	3	0	1	0	3	0	4
9		5	max	345.5	1	.513	4	.507	1	0	15	0	1	0	15
10			min	-369.277	3	.122	15	-.084	3	0	1	0	3	0	4
11		6	max	345.596	1	.475	4	.507	1	0	15	0	1	0	15
12			min	-369.205	3	.113	15	-.084	3	0	1	0	3	0	4
13		7	max	345.693	1	.437	4	.507	1	0	15	0	1	0	15
14			min	-369.132	3	.104	15	-.084	3	0	1	0	3	0	4
15		8	max	345.789	1	.399	4	.507	1	0	15	0	1	0	15
16			min	-369.06	3	.095	15	-.084	3	0	1	0	3	0	4
17		9	max	345.885	1	.362	4	.507	1	0	15	0	1	0	15
18			min	-368.988	3	.086	15	-.084	3	0	1	0	3	0	4
19		10	max	345.982	1	.324	4	.507	1	0	15	0	1	0	15
20			min	-368.915	3	.077	15	-.084	3	0	1	0	3	0	4
21		11	max	346.078	1	.286	4	.507	1	0	15	0	1	0	15
22			min	-368.843	3	.069	15	-.084	3	0	1	0	3	0	4
23		12	max	346.174	1	.248	4	.507	1	0	15	0	1	0	15
24			min	-368.771	3	.06	15	-.084	3	0	1	0	3	0	4
25		13	max	346.271	1	.21	4	.507	1	0	15	0	1	0	15
26			min	-368.699	3	.051	15	-.084	3	0	1	0	3	0	4
27		14	max	346.367	1	.172	4	.507	1	0	15	0	1	0	15
28			min	-368.626	3	.042	15	-.084	3	0	1	0	3	0	4
29		15	max	346.463	1	.135	4	.507	1	0	15	0	1	0	15
30			min	-368.554	3	.033	15	-.084	3	0	1	0	3	0	4
31		16	max	346.56	1	.097	4	.507	1	0	15	.001	1	0	15
32			min	-368.482	3	.024	15	-.084	3	0	1	0	3	0	4
33		17	max	346.656	1	.059	4	.507	1	0	15	.001	1	0	15
34			min	-368.41	3	.014	9	-.084	3	0	1	0	3	0	4
35		18	max	346.753	1	.03	10	.507	1	0	15	.001	1	0	15
36			min	-368.337	3	-.013	1	-.084	3	0	1	0	3	0	4
37		19	max	346.849	1	.006	10	.507	1	0	15	.001	1	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-368.265	3	-.042	1	-.084	3	0	1	0	3	0	4
39	M3	1	max	38.695	10	1.814	4	-.012	15	0	15	.001	1	0	4
40			min	-80.584	1	.427	15	-.388	1	0	1	0	15	0	15
41		2	max	38.639	10	1.636	4	-.012	15	0	15	.001	1	0	4
42			min	-80.651	1	.385	15	-.388	1	0	1	0	15	0	15
43		3	max	38.583	10	1.458	4	-.012	15	0	15	0	1	0	2
44			min	-80.718	1	.344	15	-.388	1	0	1	0	15	0	9
45		4	max	38.527	10	1.28	4	-.012	15	0	15	0	1	0	15
46			min	-80.785	1	.302	15	-.388	1	0	1	0	15	0	1
47		5	max	38.471	10	1.102	4	-.012	15	0	15	0	1	0	15
48			min	-80.853	1	.26	15	-.388	1	0	1	0	15	0	4
49		6	max	38.416	10	.924	4	-.012	15	0	15	0	1	0	15
50			min	-80.92	1	.218	15	-.388	1	0	1	0	15	0	4
51		7	max	38.36	10	.746	4	-.012	15	0	15	0	1	0	15
52			min	-80.987	1	.176	15	-.388	1	0	1	0	15	0	4
53		8	max	38.304	10	.568	4	-.012	15	0	15	0	1	0	15
54			min	-81.054	1	.134	15	-.388	1	0	1	0	15	0	4
55		9	max	38.248	10	.39	4	-.012	15	0	15	0	1	0	15
56			min	-81.121	1	.092	15	-.388	1	0	1	0	15	-.001	4
57		10	max	38.192	10	.212	4	-.012	15	0	15	0	1	0	15
58			min	-81.188	1	.051	15	-.388	1	0	1	0	15	-.001	4
59		11	max	38.136	10	.034	2	-.012	15	0	15	0	1	0	15
60			min	-81.255	1	.006	9	-.388	1	0	1	0	15	-.001	4
61		12	max	38.08	10	-.033	15	-.012	15	0	15	0	1	0	15
62			min	-81.322	1	-.144	4	-.388	1	0	1	0	15	-.001	4
63		13	max	38.024	10	-.075	15	-.012	15	0	15	0	1	0	15
64			min	-81.389	1	-.322	4	-.388	1	0	1	0	12	-.001	4
65		14	max	37.968	10	-.117	15	-.012	15	0	15	0	1	0	15
66			min	-81.456	1	-.5	4	-.388	1	0	1	0	3	-.001	4
67		15	max	37.912	10	-.159	15	-.012	15	0	15	0	15	0	15
68			min	-81.523	1	-.678	4	-.388	1	0	1	0	1	0	4
69		16	max	37.856	10	-.2	15	-.012	15	0	15	0	15	0	15
70			min	-81.591	1	-.856	4	-.388	1	0	1	0	1	0	4
71		17	max	37.801	10	-.242	15	-.012	15	0	15	0	15	0	15
72			min	-81.658	1	-1.034	4	-.388	1	0	1	0	1	0	4
73		18	max	37.745	10	-.284	15	-.012	15	0	15	0	15	0	15
74			min	-81.725	1	-1.212	4	-.388	1	0	1	0	1	0	4
75		19	max	37.689	10	-.326	15	-.012	15	0	15	0	15	0	1
76			min	-81.792	1	-1.39	4	-.388	1	0	1	0	1	0	1
77	M4	1	max	475.219	1	0	1	-.025	15	0	1	0	3	0	1
78			min	-116.738	3	0	1	-.755	1	0	1	0	1	0	1
79		2	max	475.283	1	0	1	-.025	15	0	1	0	15	0	1
80			min	-116.689	3	0	1	-.755	1	0	1	0	1	0	1
81		3	max	475.348	1	0	1	-.025	15	0	1	0	15	0	1
82			min	-116.641	3	0	1	-.755	1	0	1	0	1	0	1
83		4	max	475.413	1	0	1	-.025	15	0	1	0	15	0	1
84			min	-116.592	3	0	1	-.755	1	0	1	0	1	0	1
85		5	max	475.478	1	0	1	-.025	15	0	1	0	15	0	1
86			min	-116.544	3	0	1	-.755	1	0	1	0	1	0	1
87		6	max	475.542	1	0	1	-.025	15	0	1	0	15	0	1
88			min	-116.495	3	0	1	-.755	1	0	1	0	1	0	1
89		7	max	475.607	1	0	1	-.025	15	0	1	0	15	0	1
90			min	-116.447	3	0	1	-.755	1	0	1	0	1	0	1
91		8	max	475.672	1	0	1	-.025	15	0	1	0	15	0	1
92			min	-116.398	3	0	1	-.755	1	0	1	0	1	0	1
93		9	max	475.736	1	0	1	-.025	15	0	1	0	15	0	1
94			min	-116.35	3	0	1	-.755	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	475.801	1	0	1	-.025	15	0	1	0	15	0	1
96		min	-116.301	3	0	1	-.755	1	0	1	0	1	0	1
97	11	max	475.866	1	0	1	-.025	15	0	1	0	15	0	1
98		min	-116.253	3	0	1	-.755	1	0	1	0	1	0	1
99	12	max	475.931	1	0	1	-.025	15	0	1	0	15	0	1
100		min	-116.204	3	0	1	-.755	1	0	1	0	1	0	1
101	13	max	475.995	1	0	1	-.025	15	0	1	0	15	0	1
102		min	-116.156	3	0	1	-.755	1	0	1	0	1	0	1
103	14	max	476.06	1	0	1	-.025	15	0	1	0	15	0	1
104		min	-116.107	3	0	1	-.755	1	0	1	0	1	0	1
105	15	max	476.125	1	0	1	-.025	15	0	1	0	15	0	1
106		min	-116.059	3	0	1	-.755	1	0	1	0	1	0	1
107	16	max	476.189	1	0	1	-.025	15	0	1	0	15	0	1
108		min	-116.01	3	0	1	-.755	1	0	1	-.001	1	0	1
109	17	max	476.254	1	0	1	-.025	15	0	1	0	15	0	1
110		min	-115.961	3	0	1	-.755	1	0	1	-.001	1	0	1
111	18	max	476.319	1	0	1	-.025	15	0	1	0	15	0	1
112		min	-115.913	3	0	1	-.755	1	0	1	-.001	1	0	1
113	19	max	476.383	1	0	1	-.025	15	0	1	0	15	0	1
114		min	-115.864	3	0	1	-.755	1	0	1	-.001	1	0	1
115	M6	1	max	1116.218	1	.649	.215	1	0	1	0	3	0	1
116		min	-1193.702	3	.155	15	-.171	3	0	10	0	9	0	1
117	2	max	1116.315	1	.612	4	.215	1	0	1	0	3	0	15
118		min	-1193.63	3	.147	15	-.171	3	0	10	0	9	0	4
119	3	max	1116.411	1	.574	4	.215	1	0	1	0	1	0	15
120		min	-1193.558	3	.138	15	-.171	3	0	10	0	10	0	4
121	4	max	1116.507	1	.536	4	.215	1	0	1	0	1	0	15
122		min	-1193.485	3	.129	15	-.171	3	0	10	0	3	0	4
123	5	max	1116.604	1	.498	4	.215	1	0	1	0	1	0	15
124		min	-1193.413	3	.12	15	-.171	3	0	10	0	3	0	4
125	6	max	1116.7	1	.46	4	.215	1	0	1	0	1	0	15
126		min	-1193.341	3	.111	15	-.171	3	0	10	0	3	0	4
127	7	max	1116.797	1	.422	4	.215	1	0	1	0	1	0	15
128		min	-1193.268	3	.102	15	-.171	3	0	10	0	3	0	4
129	8	max	1116.893	1	.385	4	.215	1	0	1	0	1	0	15
130		min	-1193.196	3	.093	15	-.171	3	0	10	0	3	0	4
131	9	max	1116.989	1	.347	4	.215	1	0	1	0	1	0	15
132		min	-1193.124	3	.084	15	-.171	3	0	10	0	3	0	4
133	10	max	1117.086	1	.309	4	.215	1	0	1	0	1	0	15
134		min	-1193.052	3	.075	15	-.171	3	0	10	0	3	0	4
135	11	max	1117.182	1	.271	4	.215	1	0	1	0	1	0	15
136		min	-1192.979	3	.066	15	-.171	3	0	10	0	3	0	4
137	12	max	1117.278	1	.233	4	.215	1	0	1	0	1	0	15
138		min	-1192.907	3	.058	15	-.171	3	0	10	0	3	0	4
139	13	max	1117.375	1	.195	4	.215	1	0	1	0	1	0	15
140		min	-1192.835	3	.049	15	-.171	3	0	10	0	3	0	4
141	14	max	1117.471	1	.162	2	.215	1	0	1	0	1	0	15
142		min	-1192.763	3	.04	15	-.171	3	0	10	0	3	0	4
143	15	max	1117.568	1	.133	2	.215	1	0	1	0	1	0	15
144		min	-1192.69	3	.027	9	-.171	3	0	10	0	3	0	4
145	16	max	1117.664	1	.103	2	.215	1	0	1	0	1	0	15
146		min	-1192.618	3	.002	9	-.171	3	0	10	0	3	0	4
147	17	max	1117.76	1	.078	10	.215	1	0	1	0	1	0	15
148		min	-1192.546	3	-.022	9	-.171	3	0	10	0	3	0	4
149	18	max	1117.857	1	.054	10	.215	1	0	1	0	1	0	15
150		min	-1192.473	3	-.05	1	-.171	3	0	10	0	3	0	4
151	19	max	1117.953	1	.029	10	.215	1	0	1	0	1	0	15





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1192.401	3	-.079	1	-.171	3	0	10	0	3	0	4
153	M7	1	max	179.568	2	1.808	4	.008	1	0	1	0	1	4
154		min	-134.667	9	.426	15	-.008	2	0	3	0	3	0	15
155		2	max	179.501	2	1.63	4	.008	1	0	1	0	1	2
156		min	-134.723	9	.385	15	-.008	2	0	3	0	3	0	15
157		3	max	179.434	2	1.452	4	.008	1	0	1	0	1	2
158		min	-134.779	9	.343	15	-.008	2	0	3	0	3	0	9
159		4	max	179.367	2	1.274	4	.008	1	0	1	0	1	10
160		min	-134.835	9	.301	15	-.008	2	0	3	0	3	0	1
161		5	max	179.3	2	1.096	4	.008	1	0	1	0	1	15
162		min	-134.89	9	.259	15	-.008	2	0	3	0	3	0	4
163		6	max	179.233	2	.918	4	.008	1	0	1	0	1	15
164		min	-134.946	9	.217	15	-.008	2	0	3	0	3	0	4
165		7	max	179.166	2	.74	4	.008	1	0	1	0	1	15
166		min	-135.002	9	.175	15	-.008	2	0	3	0	3	0	4
167		8	max	179.098	2	.562	4	.008	1	0	1	0	1	15
168		min	-135.058	9	.134	15	-.008	2	0	3	0	3	0	4
169		9	max	179.031	2	.384	4	.008	1	0	1	0	1	15
170		min	-135.114	9	.092	15	-.008	2	0	3	0	3	-.001	4
171		10	max	178.964	2	.206	4	.008	1	0	1	0	1	15
172		min	-135.17	9	.05	15	-.008	2	0	3	0	3	-.001	4
173		11	max	178.897	2	.058	2	.008	1	0	1	0	1	15
174		min	-135.226	9	-.011	9	-.008	2	0	3	0	3	-.001	4
175		12	max	178.83	2	-.034	15	.008	1	0	1	0	1	15
176		min	-135.282	9	-.15	4	-.008	2	0	3	0	3	-.001	4
177		13	max	178.763	2	-.076	15	.008	1	0	1	0	1	15
178		min	-135.338	9	-.328	4	-.008	2	0	3	0	3	-.001	4
179		14	max	178.696	2	-.118	15	.008	1	0	1	0	1	15
180		min	-135.394	9	-.506	4	-.008	2	0	3	0	3	-.001	4
181		15	max	178.629	2	-.159	15	.008	1	0	1	0	1	15
182		min	-135.45	9	-.684	4	-.008	2	0	3	0	3	0	4
183		16	max	178.562	2	-.201	15	.008	1	0	1	0	1	15
184		min	-135.505	9	-.862	4	-.008	2	0	3	0	3	0	4
185		17	max	178.495	2	-.243	15	.008	1	0	1	0	1	15
186		min	-135.561	9	-1.041	4	-.008	2	0	3	0	3	0	4
187		18	max	178.428	2	-.285	15	.008	1	0	1	0	1	15
188		min	-135.617	9	-1.219	4	-.008	2	0	3	0	3	0	4
189		19	max	178.36	2	-.327	15	.008	1	0	1	0	1	1
190		min	-135.673	9	-1.397	4	-.008	2	0	3	0	3	0	1
191	M8	1	max	1425.043	1	0	1	.55	1	0	1	0	10	1
192		min	-402.295	3	0	1	-.285	3	0	1	0	1	0	1
193		2	max	1425.108	1	0	1	.55	1	0	1	0	1	1
194		min	-402.246	3	0	1	-.285	3	0	1	0	3	0	1
195		3	max	1425.172	1	0	1	.55	1	0	1	0	1	1
196		min	-402.198	3	0	1	-.285	3	0	1	0	3	0	1
197		4	max	1425.237	1	0	1	.55	1	0	1	0	1	1
198		min	-402.149	3	0	1	-.285	3	0	1	0	3	0	1
199		5	max	1425.302	1	0	1	.55	1	0	1	0	1	1
200		min	-402.101	3	0	1	-.285	3	0	1	0	3	0	1
201		6	max	1425.367	1	0	1	.55	1	0	1	0	1	1
202		min	-402.052	3	0	1	-.285	3	0	1	0	3	0	1
203		7	max	1425.431	1	0	1	.55	1	0	1	0	1	1
204		min	-402.004	3	0	1	-.285	3	0	1	0	3	0	1
205		8	max	1425.496	1	0	1	.55	1	0	1	0	1	1
206		min	-401.955	3	0	1	-.285	3	0	1	0	3	0	1
207		9	max	1425.561	1	0	1	.55	1	0	1	0	1	1
208		min	-401.907	3	0	1	-.285	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	1425.625	1	0	1	.55	1	0	1	0	1	0	1
210		min	-401.858	3	0	1	-.285	3	0	1	0	3	0	1
211	11	max	1425.69	1	0	1	.55	1	0	1	0	1	0	1
212		min	-401.81	3	0	1	-.285	3	0	1	0	3	0	1
213	12	max	1425.755	1	0	1	.55	1	0	1	0	1	0	1
214		min	-401.761	3	0	1	-.285	3	0	1	0	3	0	1
215	13	max	1425.82	1	0	1	.55	1	0	1	0	1	0	1
216		min	-401.713	3	0	1	-.285	3	0	1	0	3	0	1
217	14	max	1425.884	1	0	1	.55	1	0	1	0	1	0	1
218		min	-401.664	3	0	1	-.285	3	0	1	0	3	0	1
219	15	max	1425.949	1	0	1	.55	1	0	1	0	1	0	1
220		min	-401.615	3	0	1	-.285	3	0	1	0	3	0	1
221	16	max	1426.014	1	0	1	.55	1	0	1	0	1	0	1
222		min	-401.567	3	0	1	-.285	3	0	1	0	3	0	1
223	17	max	1426.078	1	0	1	.55	1	0	1	0	1	0	1
224		min	-401.518	3	0	1	-.285	3	0	1	0	3	0	1
225	18	max	1426.143	1	0	1	.55	1	0	1	0	1	0	1
226		min	-401.47	3	0	1	-.285	3	0	1	0	3	0	1
227	19	max	1426.208	1	0	1	.55	1	0	1	0	1	0	1
228		min	-401.421	3	0	1	-.285	3	0	1	0	3	0	1
229	M10	1	max 347.631	1	.653	4	-.002	15	0	1	0	2	0	1
230		min	-353.9	3	.156	15	-.073	1	0	3	0	3	0	1
231	2	max	347.727	1	.616	4	-.002	15	0	1	0	2	0	15
232		min	-353.828	3	.147	15	-.073	1	0	3	0	3	0	4
233	3	max	347.824	1	.578	4	-.002	15	0	1	0	2	0	15
234		min	-353.755	3	.138	15	-.073	1	0	3	0	3	0	4
235	4	max	347.92	1	.54	4	-.002	15	0	1	0	2	0	15
236		min	-353.683	3	.129	15	-.073	1	0	3	0	3	0	4
237	5	max	348.016	1	.502	4	-.002	15	0	1	0	15	0	15
238		min	-353.611	3	.12	15	-.073	1	0	3	0	3	0	4
239	6	max	348.113	1	.464	4	-.002	15	0	1	0	15	0	15
240		min	-353.539	3	.111	15	-.073	1	0	3	0	3	0	4
241	7	max	348.209	1	.426	4	-.002	15	0	1	0	15	0	15
242		min	-353.466	3	.103	15	-.073	1	0	3	0	3	0	4
243	8	max	348.306	1	.389	4	-.002	15	0	1	0	15	0	15
244		min	-353.394	3	.094	15	-.073	1	0	3	0	3	0	4
245	9	max	348.402	1	.351	4	-.002	15	0	1	0	15	0	15
246		min	-353.322	3	.085	15	-.073	1	0	3	0	3	0	4
247	10	max	348.498	1	.313	4	-.002	15	0	1	0	15	0	15
248		min	-353.249	3	.076	15	-.073	1	0	3	0	3	0	4
249	11	max	348.595	1	.275	4	-.002	15	0	1	0	15	0	15
250		min	-353.177	3	.067	15	-.073	1	0	3	0	3	0	4
251	12	max	348.691	1	.237	4	-.002	15	0	1	0	15	0	15
252		min	-353.105	3	.058	15	-.073	1	0	3	0	3	0	4
253	13	max	348.787	1	.199	4	-.002	15	0	1	0	15	0	15
254		min	-353.033	3	.049	15	-.073	1	0	3	0	3	0	4
255	14	max	348.884	1	.162	4	-.002	15	0	1	0	15	0	15
256		min	-352.96	3	.04	15	-.073	1	0	3	0	3	0	4
257	15	max	348.98	1	.124	4	-.002	15	0	1	0	15	0	15
258		min	-352.888	3	.031	15	-.073	1	0	3	0	3	0	4
259	16	max	349.076	1	.101	3	-.002	15	0	1	0	15	0	15
260		min	-352.816	3	.013	9	-.073	1	0	3	0	3	0	4
261	17	max	349.173	1	.079	3	-.002	15	0	1	0	15	0	15
262		min	-352.744	3	-.012	9	-.073	1	0	3	0	3	0	4
263	18	max	349.269	1	.057	3	-.002	15	0	1	0	15	0	15
264		min	-352.671	3	-.036	9	-.073	1	0	3	0	3	0	4
265	19	max	349.366	1	.035	3	-.002	15	0	1	0	15	0	15



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-352.599	3	-.061	9	-.073	1	0	3	0	3	0	4
267		1	max	38.212	10	1.818	4	.444	1	0	1	0	3	0	4
268			min	-80.46	1	.428	15	.004	12	0	15	-.001	1	0	15
269		2	max	38.156	10	1.64	4	.444	1	0	1	0	3	0	4
270			min	-80.527	1	.386	15	.004	12	0	15	-.001	1	0	15
271		3	max	38.1	10	1.462	4	.444	1	0	1	0	3	0	4
272			min	-80.594	1	.344	15	.004	12	0	15	0	1	0	3
273		4	max	38.045	10	1.284	4	.444	1	0	1	0	3	0	15
274			min	-80.661	1	.302	15	.004	12	0	15	0	1	0	1
275		5	max	37.989	10	1.106	4	.444	1	0	1	0	3	0	15
276			min	-80.728	1	.26	15	.004	12	0	15	0	1	0	4
277		6	max	37.933	10	.928	4	.444	1	0	1	0	3	0	15
278			min	-80.795	1	.219	15	.004	12	0	15	0	1	0	4
279		7	max	37.877	10	.75	4	.444	1	0	1	0	3	0	15
280			min	-80.862	1	.177	15	.004	12	0	15	0	1	0	4
281		8	max	37.821	10	.572	4	.444	1	0	1	0	3	0	15
282			min	-80.929	1	.135	15	.004	12	0	15	0	1	0	4
283	9	max	37.765	10	.394	4	.444	1	0	1	0	3	0	15	
284		min	-80.996	1	.093	15	.004	12	0	15	0	1	-.001	4	
285	10	max	37.709	10	.216	4	.444	1	0	1	0	3	0	15	
286		min	-81.063	1	.051	15	.004	12	0	15	0	1	-.001	4	
287	11	max	37.653	10	.038	4	.444	1	0	1	0	3	0	15	
288		min	-81.131	1	.001	3	.004	12	0	15	0	1	-.001	4	
289	12	max	37.597	10	-.032	15	.444	1	0	1	0	3	0	15	
290		min	-81.198	1	-.14	4	.004	12	0	15	0	1	-.001	4	
291	13	max	37.541	10	-.074	15	.444	1	0	1	0	3	0	15	
292		min	-81.265	1	-.318	4	.004	12	0	15	0	10	-.001	4	
293	14	max	37.485	10	-.116	15	.444	1	0	1	0	3	0	15	
294		min	-81.332	1	-.496	4	.004	12	0	15	0	10	-.001	4	
295	15	max	37.429	10	-.158	15	.444	1	0	1	0	3	0	15	
296		min	-81.399	1	-.674	4	.004	12	0	15	0	15	0	4	
297	16	max	37.374	10	-.2	15	.444	1	0	1	0	1	0	15	
298		min	-81.466	1	-.852	4	.004	12	0	15	0	15	0	4	
299	17	max	37.318	10	-.242	15	.444	1	0	1	0	1	0	15	
300		min	-81.533	1	-1.03	4	.004	12	0	15	0	15	0	4	
301	18	max	37.262	10	-.284	15	.444	1	0	1	0	1	0	15	
302		min	-81.6	1	-1.208	4	.004	12	0	15	0	15	0	4	
303	19	max	37.206	10	-.325	15	.444	1	0	1	0	1	0	1	
304		min	-81.667	1	-1.386	4	.004	12	0	15	0	15	0	1	
305	M12	1	max	475.184	1	0	1	1.879	1	0	1	0	1	0	1
306		min	-116.373	3	0	1	.058	15	0	1	0	3	0	1	
307	2	max	475.248	1	0	1	1.879	1	0	1	0	1	0	1	
308		min	-116.325	3	0	1	.058	15	0	1	0	15	0	1	
309	3	max	475.313	1	0	1	1.879	1	0	1	0	1	0	1	
310		min	-116.276	3	0	1	.058	15	0	1	0	15	0	1	
311	4	max	475.378	1	0	1	1.879	1	0	1	0	1	0	1	
312		min	-116.228	3	0	1	.058	15	0	1	0	15	0	1	
313	5	max	475.442	1	0	1	1.879	1	0	1	0	1	0	1	
314		min	-116.179	3	0	1	.058	15	0	1	0	15	0	1	
315	6	max	475.507	1	0	1	1.879	1	0	1	0	1	0	1	
316		min	-116.131	3	0	1	.058	15	0	1	0	15	0	1	
317	7	max	475.572	1	0	1	1.879	1	0	1	.001	1	0	1	
318		min	-116.082	3	0	1	.058	15	0	1	0	15	0	1	
319	8	max	475.637	1	0	1	1.879	1	0	1	.001	1	0	1	
320		min	-116.034	3	0	1	.058	15	0	1	0	15	0	1	
321	9	max	475.701	1	0	1	1.879	1	0	1	.001	1	0	1	
322		min	-115.985	3	0	1	.058	15	0	1	0	15	0	1	





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380			min	-85.225	1	-164.675	3	-37.329	1	0	1	-.072	1	-.009	3
381	M5	1	max	194.15	1	1133.653	3	0	10	0	1	.004	3	.026	3
382			min	4.299	12	-1132.69	1	-32.345	3	0	3	0	10	-.03	1
383		2	max	194.222	1	1133.451	3	0	10	0	1	0	1	.215	1
384			min	4.335	12	-1132.959	1	-32.345	3	0	3	-.003	3	-.22	3
385		3	max	233.764	1	8.422	9	3.605	3	0	3	0	1	.457	1
386			min	-35.744	3	-75.645	3	-.579	1	0	1	-.009	3	-.461	3
387		4	max	233.836	1	8.197	9	3.605	3	0	3	0	1	.46	1
388			min	-35.689	3	-75.848	3	-.579	1	0	1	-.009	3	-.445	3
389		5	max	233.908	1	7.972	9	3.605	3	0	3	0	1	.464	1
390			min	-35.635	3	-76.05	3	-.579	1	0	1	-.008	3	-.428	3
391		6	max	233.98	1	7.748	9	3.605	3	0	3	0	1	.468	1
392			min	-35.581	3	-76.252	3	-.579	1	0	1	-.007	3	-.412	3
393		7	max	234.053	1	7.523	9	3.605	3	0	3	0	1	.472	1
394			min	-35.527	3	-76.454	3	-.579	1	0	1	-.006	3	-.395	3
395		8	max	234.125	1	7.298	9	3.605	3	0	3	0	1	.476	1
396			min	-35.473	3	-76.657	3	-.579	1	0	1	-.006	3	-.378	3
397		9	max	234.197	1	7.073	9	3.605	3	0	3	0	1	.48	1
398			min	-35.418	3	-76.859	3	-.579	1	0	1	-.005	3	-.362	3
399		10	max	234.269	1	6.849	9	3.605	3	0	3	0	10	.484	1
400			min	-35.364	3	-77.061	3	-.579	1	0	1	-.004	3	-.345	3
401		11	max	234.342	1	6.624	9	3.605	3	0	3	0	10	.488	1
402			min	-35.31	3	-77.264	3	-.579	1	0	1	-.003	3	-.328	3
403		12	max	234.414	1	6.399	9	3.605	3	0	3	0	10	.492	1
404			min	-35.256	3	-77.466	3	-.579	1	0	1	-.002	3	-.312	3
405		13	max	234.486	1	6.174	9	3.605	3	0	3	0	10	.497	1
406			min	-35.202	3	-77.668	3	-.579	1	0	1	-.002	3	-.295	3
407		14	max	234.559	1	5.95	9	3.605	3	0	3	0	10	.501	1
408			min	-35.147	3	-77.871	3	-.579	1	0	1	0	3	-.278	3
409		15	max	234.631	1	5.725	9	3.605	3	0	3	0	10	.505	1
410			min	-35.093	3	-78.073	3	-.579	1	0	1	0	1	-.261	3
411		16	max	242.403	2	61.293	2	3.582	3	0	1	0	3	.515	2
412			min	-111.62	3	-144.624	3	-.564	1	0	10	0	1	-.243	3
413		17	max	242.476	2	61.023	2	3.582	3	0	1	.001	3	.526	1
414			min	-111.566	3	-144.827	3	-.564	1	0	10	0	1	-.212	3
415		18	max	-5.189	12	1300.002	1	3.293	3	0	3	.002	3	.249	1
416			min	-194.279	1	-534.421	3	-.133	1	0	1	0	1	-.098	3
417		19	max	-5.153	12	1299.732	1	3.293	3	0	3	.003	3	.018	3
418			min	-194.207	1	-534.623	3	-.133	1	0	1	0	1	-.033	1
419	M9	1	max	84.95	1	346.235	3	37.18	1	0	3	-.002	15	.015	1
420			min	2.614	15	-346.447	1	1.289	15	0	1	-.071	1	-.013	3
421		2	max	85.022	1	346.033	3	37.18	1	0	3	0	12	.09	1
422			min	2.636	15	-346.716	1	1.289	15	0	1	-.062	1	-.088	3
423		3	max	98.125	1	5.593	9	35.229	1	0	1	.006	3	.164	1
424			min	-7.118	3	-23.462	3	-.659	3	0	15	-.054	1	-.161	3
425		4	max	98.197	1	5.368	9	35.229	1	0	1	.006	3	.164	1
426			min	-7.064	3	-23.665	3	-.659	3	0	15	-.046	1	-.156	3
427		5	max	98.27	1	5.143	9	35.229	1	0	1	.006	3	.165	1
428			min	-7.01	3	-23.867	3	-.659	3	0	15	-.038	1	-.151	3
429		6	max	98.342	1	4.918	9	35.229	1	0	1	.006	3	.165	1
430			min	-6.955	3	-24.069	3	-.659	3	0	15	-.031	1	-.146	3
431		7	max	98.414	1	4.694	9	35.229	1	0	1	.005	3	.166	1
432			min	-6.901	3	-24.272	3	-.659	3	0	15	-.023	1	-.141	3
433		8	max	98.486	1	4.469	9	35.229	1	0	1	.005	3	.167	1
434			min	-6.847	3	-24.474	3	-.659	3	0	15	-.015	1	-.135	3
435		9	max	98.559	1	4.244	9	35.229	1	0	1	.005	3	.167	1
436			min	-6.793	3	-24.676	3	-.659	3	0	15	-.008	1	-.13	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	98.631	1	4.019	9	35.229	1	0	1	.005	3	.168	1
438			min	-6.739	3	-24.878	3	-.659	3	0	15	0	1	-.125	3
439		11	max	98.703	1	3.794	9	35.229	1	0	1	.008	1	.169	1
440			min	-6.684	3	-25.081	3	-.659	3	0	15	0	15	-.119	3
441		12	max	98.775	1	3.57	9	35.229	1	0	1	.015	1	.17	1
442			min	-6.63	3	-25.283	3	-.659	3	0	15	0	15	-.114	3
443		13	max	98.848	1	3.345	9	35.229	1	0	1	.023	1	.171	1
444			min	-6.576	3	-25.485	3	-.659	3	0	15	0	15	-.108	3
445		14	max	98.92	1	3.12	9	35.229	1	0	1	.03	1	.172	1
446			min	-6.522	3	-25.688	3	-.659	3	0	15	0	15	-.103	3
447		15	max	98.992	1	2.895	9	35.229	1	0	1	.038	1	.175	2
448			min	-6.468	3	-25.89	3	-.659	3	0	15	.001	15	-.097	3
449		16	max	69.425	2	11.178	10	35.647	1	0	15	.047	1	.179	2
450			min	-34.764	3	-59.335	1	-.647	3	0	1	.001	15	-.091	3
451		17	max	69.497	2	10.953	10	35.647	1	0	15	.054	1	.187	1
452			min	-34.71	3	-59.605	1	-.647	3	0	1	.002	15	-.08	3
453		18	max	-2.634	15	398.519	1	37.471	1	0	1	.062	1	.103	1
454			min	-84.994	1	-164.471	3	-.376	3	0	3	.002	15	-.045	3
455		19	max	-2.612	15	398.249	1	37.471	1	0	1	.071	1	.016	1
456			min	-84.921	1	-164.673	3	-.376	3	0	3	.002	15	-.009	3
457	M13	1	max	37.257	1	346.08	1	-2.614	15	.015	1	.071	1	0	1
458			min	1.289	15	-346.236	3	-84.945	1	-.013	3	.002	15	0	3
459		2	max	37.257	1	244.843	1	-1.994	15	.015	1	.019	1	.205	3
460			min	1.289	15	-244.789	3	-64.703	1	-.013	3	0	10	-.205	1
461		3	max	37.257	1	143.605	1	-1.375	15	.015	1	.003	3	.34	3
462			min	1.289	15	-143.343	3	-44.461	1	-.013	3	-.019	1	-.34	1
463		4	max	37.257	1	42.368	1	-.755	15	.015	1	.001	3	.404	3
464			min	1.289	15	-41.897	3	-24.219	1	-.013	3	-.043	1	-.405	1
465		5	max	37.257	1	59.55	3	.134	10	.015	1	0	3	.398	3
466			min	1.289	15	-58.869	1	-3.977	1	-.013	3	-.053	1	-.399	1
467		6	max	37.257	1	160.996	3	16.264	1	.015	1	0	12	.322	3
468			min	1.289	15	-160.107	1	-.398	3	-.013	3	-.049	1	-.323	1
469		7	max	37.257	1	262.443	3	36.506	1	.015	1	0	12	.175	3
470			min	1.289	15	-261.344	1	.412	12	-.013	3	-.03	1	-.177	1
471		8	max	37.257	1	363.889	3	56.748	1	.015	1	.002	2	.04	1
472			min	1.289	15	-362.581	1	1.017	12	-.013	3	0	15	-.043	3
473		9	max	37.257	1	465.335	3	76.99	1	.015	1	.048	1	.327	1
474			min	1.289	15	-463.818	1	1.621	12	-.013	3	.001	12	-.331	3
475		10	max	37.257	1	566.782	3	97.232	1	.015	1	.109	1	.684	1
476			min	1.289	15	-565.056	1	2.226	12	-.013	3	.002	12	-.689	3
477		11	max	36.589	1	463.818	1	-1.467	12	.013	3	.048	1	.327	1
478			min	1.143	15	-465.335	3	-76.686	1	-.015	1	-.003	3	-.331	3
479		12	max	36.589	1	362.581	1	-.863	12	.013	3	.002	2	.04	1
480			min	1.143	15	-363.889	3	-56.445	1	-.015	1	-.004	3	-.043	3
481		13	max	36.589	1	261.344	1	-.254	3	.013	3	0	15	.175	3
482			min	1.143	15	-262.443	3	-36.203	1	-.015	1	-.031	1	-.177	1
483		14	max	36.589	1	160.106	1	.652	3	.013	3	-.002	15	.322	3
484			min	1.143	15	-160.996	3	-15.961	1	-.015	1	-.049	1	-.323	1
485		15	max	36.589	1	58.869	1	4.281	1	.013	3	-.002	15	.398	3
486			min	1.143	15	-59.55	3	-.134	10	-.015	1	-.053	1	-.399	1
487		16	max	36.589	1	41.897	3	24.523	1	.013	3	-.001	12	.404	3
488			min	1.143	15	-42.368	1	.767	15	-.015	1	-.043	1	-.405	1
489		17	max	36.589	1	143.343	3	44.765	1	.013	3	0	3	.34	3
490			min	1.143	15	-143.605	1	1.386	15	-.015	1	-.019	1	-.34	1
491		18	max	36.589	1	244.789	3	65.007	1	.013	3	.019	1	.205	3
492			min	1.143	15	-244.843	1	2.006	15	-.015	1	0	10	-.205	1
493		19	max	36.589	1	346.236	3	85.249	1	.013	3	.072	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	1.143	15	-346.08	1	2.625	15	-.015	1	.002	15	0	3
495	M16	1	max	.377	3	398.635	1	-2.612	15	.009	3	.071	1	0	1
496			min	-37.394	1	-164.687	3	-84.927	1	-.016	1	.002	15	0	3
497		2	max	.377	3	281.981	1	-1.992	15	.009	3	.019	1	.098	3
498			min	-37.394	1	-116.683	3	-64.685	1	-.016	1	0	10	-.236	1
499		3	max	.377	3	165.327	1	-1.373	15	.009	3	0	12	.162	3
500			min	-37.394	1	-68.68	3	-44.443	1	-.016	1	-.019	1	-.392	1
501		4	max	.377	3	48.674	1	-.753	15	.009	3	-.001	15	.193	3
502			min	-37.394	1	-20.677	3	-24.201	1	-.016	1	-.043	1	-.466	1
503		5	max	.377	3	27.327	3	.137	10	.009	3	-.002	15	.191	3
504			min	-37.394	1	-67.98	1	-3.96	1	-.016	1	-.053	1	-.459	1
505		6	max	.377	3	75.33	3	16.282	1	.009	3	-.002	15	.155	3
506			min	-37.394	1	-184.633	1	.02	3	-.016	1	-.049	1	-.372	1
507		7	max	.377	3	123.333	3	36.524	1	.009	3	0	15	.086	3
508			min	-37.394	1	-301.287	1	.663	12	-.016	1	-.03	1	-.203	1
509		8	max	.377	3	171.337	3	56.766	1	.009	3	.002	2	.047	1
510			min	-37.394	1	-417.941	1	1.268	12	-.016	1	-.002	3	-.016	3
511		9	max	.377	3	219.34	3	77.008	1	.009	3	.049	1	.378	1
512			min	-37.394	1	-534.594	1	1.872	12	-.016	1	0	3	-.152	3
513		10	max	-1.165	15	-12.906	15	97.25	1	0	15	.109	1	.789	1
514			min	-37.394	1	-651.248	1	-3.982	3	-.016	1	.003	12	-.321	3
515		11	max	-1.165	15	534.594	1	-2.072	12	.016	1	.048	1	.378	1
516			min	-37.257	1	-219.34	3	-76.704	1	-.009	3	.001	12	-.152	3
517		12	max	-1.165	15	417.941	1	-1.468	12	.016	1	.002	2	.047	1
518			min	-37.257	1	-171.337	3	-56.463	1	-.009	3	0	3	-.016	3
519		13	max	-1.165	15	301.287	1	-.863	12	.016	1	0	12	.086	3
520			min	-37.257	1	-123.333	3	-36.221	1	-.009	3	-.031	1	-.203	1
521		14	max	-1.165	15	184.633	1	-.259	12	.016	1	-.001	12	.155	3
522			min	-37.257	1	-75.33	3	-15.979	1	-.009	3	-.049	1	-.372	1
523		15	max	-1.165	15	67.98	1	4.263	1	.016	1	-.001	12	.191	3
524			min	-37.257	1	-27.327	3	-.137	10	-.009	3	-.053	1	-.459	1
525		16	max	-1.165	15	20.677	3	24.505	1	.016	1	0	12	.193	3
526			min	-37.257	1	-48.674	1	.762	15	-.009	3	-.043	1	-.466	1
527		17	max	-1.165	15	68.68	3	44.747	1	.016	1	0	3	.162	3
528			min	-37.257	1	-165.327	1	1.382	15	-.009	3	-.019	1	-.392	1
529		18	max	-1.165	15	116.683	3	64.989	1	.016	1	.019	1	.098	3
530			min	-37.257	1	-281.981	1	2.001	15	-.009	3	0	10	-.236	1
531		19	max	-1.165	15	164.687	3	85.231	1	.016	1	.072	1	0	1
532			min	-37.257	1	-398.635	1	2.621	15	-.009	3	.002	15	0	3
533	M15	1	max	.173	13	2.18	4	.046	3	0	9	0	9	0	1
534			min	-38.544	3	0	1	-.024	9	0	3	0	3	0	1
535		2	max	.099	13	1.938	4	.046	3	0	9	0	9	0	1
536			min	-38.598	3	0	1	-.024	9	0	3	0	3	0	4
537		3	max	.025	13	1.696	4	.046	3	0	9	0	9	0	1
538			min	-38.652	3	0	1	-.024	9	0	3	0	3	-.001	4
539		4	max	0	1	1.454	4	.046	3	0	9	0	9	0	1
540			min	-38.706	3	0	1	-.024	9	0	3	0	3	-.002	4
541		5	max	0	1	1.211	4	.046	3	0	9	0	9	0	1
542			min	-38.76	3	0	1	-.024	9	0	3	0	3	-.003	4
543		6	max	0	1	.969	4	.046	3	0	9	0	9	0	1
544			min	-38.814	3	0	1	-.024	9	0	3	0	3	-.003	4
545		7	max	0	1	.727	4	.046	3	0	9	0	3	0	1
546			min	-38.868	3	0	1	-.024	9	0	3	0	9	-.003	4
547		8	max	0	1	.485	4	.046	3	0	9	0	3	0	1
548			min	-38.922	3	0	1	-.024	9	0	3	0	9	-.003	4
549		9	max	0	1	.242	4	.046	3	0	9	0	3	0	1
550			min	-38.976	3	0	1	-.024	9	0	3	0	9	-.004	4



Company : Schletter, Inc.  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.046	3	0	9	0	3	0	1
552		min	-39.03	3	0	1	-.024	9	0	3	0	9	-.004	4
553	11	max	0	1	0	1	.046	3	0	9	0	3	0	1
554		min	-39.084	3	-.242	4	-.024	9	0	3	0	9	-.004	4
555	12	max	0	1	0	1	.046	3	0	9	0	3	0	1
556		min	-39.138	3	-.485	4	-.024	9	0	3	0	9	-.003	4
557	13	max	0	1	0	1	.046	3	0	9	0	3	0	1
558		min	-39.192	3	-.727	4	-.024	9	0	3	0	9	-.003	4
559	14	max	0	1	0	1	.046	3	0	9	0	3	0	1
560		min	-39.246	3	-.969	4	-.024	9	0	3	0	9	-.003	4
561	15	max	0	1	0	1	.046	3	0	9	0	3	0	1
562		min	-39.3	3	-1.211	4	-.024	9	0	3	0	9	-.003	4
563	16	max	0	1	0	1	.046	3	0	9	0	3	0	1
564		min	-39.354	3	-1.454	4	-.024	9	0	3	0	9	-.002	4
565	17	max	0	1	0	1	.046	3	0	9	0	3	0	1
566		min	-39.408	3	-1.696	4	-.024	9	0	3	0	9	-.001	4
567	18	max	0	1	0	1	.046	3	0	9	0	3	0	1
568		min	-39.462	3	-1.938	4	-.024	9	0	3	0	9	0	4
569	19	max	0	1	0	1	.046	3	0	9	0	3	0	1
570		min	-39.516	3	-2.18	4	-.024	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.18	.028	1	0	3	0	3	0	1
572		min	-38.804	3	0	10	-.02	3	0	1	0	1	0	1
573	2	max	0	10	1.938	4	.028	1	0	3	0	3	0	10
574		min	-38.75	3	0	10	-.02	3	0	1	0	1	0	4
575	3	max	0	10	1.696	4	.028	1	0	3	0	3	0	10
576		min	-38.696	3	0	10	-.02	3	0	1	0	1	-.001	4
577	4	max	0	10	1.454	4	.028	1	0	3	0	3	0	10
578		min	-38.643	3	0	10	-.02	3	0	1	0	1	-.002	4
579	5	max	0	10	1.211	4	.028	1	0	3	0	3	0	10
580		min	-38.589	3	0	10	-.02	3	0	1	0	1	-.003	4
581	6	max	0	10	.969	4	.028	1	0	3	0	3	0	10
582		min	-38.535	3	0	10	-.02	3	0	1	0	1	-.003	4
583	7	max	0	10	.727	4	.028	1	0	3	0	3	0	10
584		min	-38.481	3	0	10	-.02	3	0	1	0	1	-.003	4
585	8	max	0	10	.485	4	.028	1	0	3	0	3	0	10
586		min	-38.427	3	0	10	-.02	3	0	1	0	1	-.003	4
587	9	max	0	10	.242	4	.028	1	0	3	0	3	0	10
588		min	-38.373	3	0	10	-.02	3	0	1	0	1	-.004	4
589	10	max	0	10	0	1	.028	1	0	3	0	3	0	10
590		min	-38.319	3	0	1	-.02	3	0	1	0	1	-.004	4
591	11	max	0	10	0	10	.028	1	0	3	0	3	0	10
592		min	-38.265	3	-.242	4	-.02	3	0	1	0	1	-.004	4
593	12	max	0	10	0	10	.028	1	0	3	0	3	0	10
594		min	-38.211	3	-.485	4	-.02	3	0	1	0	1	-.003	4
595	13	max	0	10	0	10	.028	1	0	3	0	2	0	10
596		min	-38.157	3	-.727	4	-.02	3	0	1	0	4	-.003	4
597	14	max	0	10	0	10	.028	1	0	3	0	1	0	10
598		min	-38.103	3	-.969	4	-.02	3	0	1	0	3	-.003	4
599	15	max	0	10	0	10	.028	1	0	3	0	1	0	10
600		min	-38.049	3	-1.211	4	-.02	3	0	1	0	3	-.003	4
601	16	max	0	10	0	10	.028	1	0	3	0	1	0	10
602		min	-37.995	3	-1.454	4	-.02	3	0	1	0	3	-.002	4
603	17	max	.02	2	0	10	.028	1	0	3	0	1	0	10
604		min	-37.941	3	-1.696	4	-.02	3	0	1	0	3	-.001	4
605	18	max	.092	2	0	10	.028	1	0	3	0	1	0	10
606		min	-37.887	3	-1.938	4	-.02	3	0	1	0	3	0	4
607	19	max	.164	2	0	10	.028	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-37.833	3	-2.18	4	-0.02	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.007	1	-1.573e-5	15	NC	3	NC	2	
2			min	-.003	3	-.005	3	0	3	-5.003e-4	1	4848.01	2	4043.719	1	
3			2	max	.002	1	.006	2	.007	1	-1.512e-5	15	NC	3	NC	2
4				min	-.003	3	-.005	3	0	3	-4.812e-4	1	5248.787	2	4385.058	1
5			3	max	.002	1	.005	2	.006	1	-1.451e-5	15	NC	3	NC	2
6				min	-.002	3	-.005	3	0	3	-4.622e-4	1	5718.431	2	4786.91	1
7			4	max	.002	1	.005	2	.006	1	-1.39e-5	15	NC	3	NC	2
8				min	-.002	3	-.004	3	0	3	-4.431e-4	1	6272.479	2	5264.088	1
9			5	max	.002	1	.004	2	.005	1	-1.329e-5	15	NC	3	NC	2
10				min	-.002	3	-.004	3	0	3	-4.241e-4	1	6931.345	2	5836.381	1
11		6	max	.002	1	.004	2	.005	1	-1.269e-5	15	NC	1	NC	2	
12			min	-.002	3	-.004	3	0	3	-4.05e-4	1	7722.276	2	6530.712	1	
13		7	max	.002	1	.003	2	.004	1	-1.208e-5	15	NC	1	NC	2	
14			min	-.002	3	-.004	3	0	3	-3.859e-4	1	8682.33	2	7384.488	1	
15		8	max	.002	1	.003	2	.004	1	-1.147e-5	15	NC	1	NC	2	
16			min	-.002	3	-.004	3	0	3	-3.669e-4	1	9863.017	2	8450.955	1	
17		9	max	.001	1	.003	2	.003	1	-1.086e-5	15	NC	1	NC	2	
18			min	-.002	3	-.003	3	0	3	-3.478e-4	1	NC	1	9808.073	1	
19		10	max	.001	1	.002	2	.003	1	-1.025e-5	15	NC	1	NC	1	
20			min	-.001	3	-.003	3	0	3	-3.287e-4	1	NC	1	NC	1	
21		11	max	.001	1	.002	2	.002	1	-9.642e-6	15	NC	1	NC	1	
22			min	-.001	3	-.003	3	0	3	-3.097e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	-9.033e-6	15	NC	1	NC	1	
24			min	-.001	3	-.003	3	0	3	-2.906e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	.001	1	-8.424e-6	15	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-2.715e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.001	1	-7.815e-6	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-2.525e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-7.207e-6	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-2.334e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-6.598e-6	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-2.143e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-5.989e-6	15	NC	1	NC	1	
34			min	0	3	0	3	0	3	-1.953e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-5.38e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	3	-1.762e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-4.679e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.571e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	7.134e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	2.167e-6	15	NC	1	NC	1	
41			2	max	0	1	0	2	0	12	8.983e-5	1	NC	1	NC	1
42				min	0	10	0	3	0	1	2.748e-6	15	NC	1	NC	1
43			3	max	0	1	0	2	0	12	1.083e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	0	1	3.329e-6	15	NC	1	NC	1
45			4	max	0	1	0	2	0	12	1.268e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	0	1	3.909e-6	15	NC	1	NC	1
47			5	max	0	1	0	2	0	3	1.453e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	0	1	4.49e-6	15	NC	1	NC	1
49			6	max	0	1	0	2	0	3	1.638e-4	1	NC	1	NC	1
50				min	0	10	-.003	3	0	1	5.071e-6	15	NC	1	NC	1
51		7	max	0	1	0	2	0	3	1.823e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	10	-.004	3	0	1	5.651e-6	15	NC	1	NC	1
53		8	max	0	1	.001	2	0	3	2.007e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	6.232e-6	15	NC	1	NC	1
55		9	max	0	1	.002	2	0	3	2.192e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	1	6.813e-6	15	NC	1	NC	1
57		10	max	0	1	.002	2	0	2	2.377e-4	1	NC	1	NC	1
58			min	0	10	-.006	3	0	15	7.393e-6	15	NC	1	NC	1
59		11	max	0	1	.002	2	0	1	2.562e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	15	7.974e-6	15	NC	1	NC	1
61		12	max	0	1	.003	2	0	1	2.747e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	15	8.555e-6	15	NC	1	NC	1
63		13	max	0	1	.004	2	.001	1	2.932e-4	1	NC	1	NC	1
64			min	0	10	-.007	3	0	15	9.135e-6	15	NC	1	NC	1
65		14	max	0	1	.005	2	.002	1	3.116e-4	1	NC	1	NC	1
66			min	0	10	-.007	3	0	15	9.716e-6	15	NC	1	NC	1
67		15	max	0	1	.005	2	.002	1	3.301e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	15	1.03e-5	15	8516.468	2	NC	1
69		16	max	0	1	.006	2	.002	1	3.486e-4	1	NC	3	NC	1
70			min	0	10	-.007	3	0	15	1.088e-5	15	7270.682	2	NC	1
71		17	max	0	1	.007	2	.003	1	3.671e-4	1	NC	3	NC	1
72			min	0	10	-.007	3	0	15	1.146e-5	15	6294.299	2	NC	1
73		18	max	0	1	.008	2	.003	1	3.856e-4	1	NC	3	NC	1
74			min	0	10	-.007	3	0	15	1.204e-5	15	5522.322	2	NC	1
75		19	max	0	1	.009	2	.003	1	4.041e-4	1	NC	3	NC	1
76			min	0	10	-.007	3	0	15	1.262e-5	15	4907.747	2	NC	1
77	M4	1	max	.002	1	.007	2	0	15	-1.433e-5	15	NC	1	NC	2
78			min	0	3	-.005	3	-.002	1	-4.66e-4	1	NC	1	7927.187	1
79		2	max	.002	1	.007	2	0	15	-1.433e-5	15	NC	1	NC	2
80			min	0	3	-.005	3	-.002	1	-4.66e-4	1	NC	1	8648.087	1
81		3	max	.002	1	.006	2	0	15	-1.433e-5	15	NC	1	NC	2
82			min	0	3	-.005	3	-.002	1	-4.66e-4	1	NC	1	9506.062	1
83		4	max	.002	1	.006	2	0	15	-1.433e-5	15	NC	1	NC	1
84			min	0	3	-.004	3	-.002	1	-4.66e-4	1	NC	1	NC	1
85		5	max	.002	1	.006	2	0	15	-1.433e-5	15	NC	1	NC	1
86			min	0	3	-.004	3	-.002	1	-4.66e-4	1	NC	1	NC	1
87		6	max	.002	1	.005	2	0	15	-1.433e-5	15	NC	1	NC	1
88			min	0	3	-.004	3	-.001	1	-4.66e-4	1	NC	1	NC	1
89		7	max	.002	1	.005	2	0	15	-1.433e-5	15	NC	1	NC	1
90			min	0	3	-.004	3	-.001	1	-4.66e-4	1	NC	1	NC	1
91		8	max	.001	1	.004	2	0	15	-1.433e-5	15	NC	1	NC	1
92			min	0	3	-.003	3	-.001	1	-4.66e-4	1	NC	1	NC	1
93		9	max	.001	1	.004	2	0	15	-1.433e-5	15	NC	1	NC	1
94			min	0	3	-.003	3	0	1	-4.66e-4	1	NC	1	NC	1
95		10	max	.001	1	.004	2	0	15	-1.433e-5	15	NC	1	NC	1
96			min	0	3	-.003	3	0	1	-4.66e-4	1	NC	1	NC	1
97		11	max	.001	1	.003	2	0	15	-1.433e-5	15	NC	1	NC	1
98			min	0	3	-.002	3	0	1	-4.66e-4	1	NC	1	NC	1
99		12	max	0	1	.003	2	0	15	-1.433e-5	15	NC	1	NC	1
100			min	0	3	-.002	3	0	1	-4.66e-4	1	NC	1	NC	1
101		13	max	0	1	.002	2	0	15	-1.433e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-4.66e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-1.433e-5	15	NC	1	NC	1
104			min	0	3	-.001	3	0	1	-4.66e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-1.433e-5	15	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-4.66e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-1.433e-5	15	NC	1	NC	1
108			min	0	3	0	3	0	1	-4.66e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-1.433e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-4.66e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.433e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-4.66e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.433e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-4.66e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.018	2	.003	1	1.922e-4	3	NC	3	NC	2
116			min	-.009	3	-.013	3	-.003	3	-6.478e-8	10	1642.072	2	9515.421	1
117		2	max	.008	1	.017	2	.003	1	1.879e-4	3	NC	3	NC	1
118			min	-.008	3	-.012	3	-.002	3	-6.137e-8	10	1752.238	2	NC	1
119		3	max	.007	1	.016	2	.003	1	1.836e-4	3	NC	3	NC	1
120			min	-.008	3	-.012	3	-.002	3	-5.796e-8	10	1877.877	2	NC	1
121		4	max	.007	1	.015	2	.002	1	1.792e-4	3	NC	3	NC	1
122			min	-.007	3	-.011	3	-.002	3	-5.455e-8	10	2022.078	2	NC	1
123		5	max	.006	1	.014	2	.002	1	1.749e-4	3	NC	3	NC	1
124			min	-.007	3	-.011	3	-.002	3	-8.973e-8	2	2188.824	2	NC	1
125		6	max	.006	1	.013	2	.002	1	1.706e-4	3	NC	3	NC	1
126			min	-.006	3	-.01	3	-.002	3	-1.925e-6	2	2383.324	2	NC	1
127		7	max	.006	1	.012	2	.002	1	1.663e-4	3	NC	3	NC	1
128			min	-.006	3	-.009	3	-.002	3	-3.76e-6	2	2612.53	2	NC	1
129		8	max	.005	1	.01	2	.002	1	1.62e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.001	3	-6.335e-6	1	2885.925	2	NC	1
131		9	max	.005	1	.009	2	.001	1	1.576e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	-.001	3	-1.216e-5	1	3216.792	2	NC	1
133		10	max	.004	1	.008	2	.001	1	1.533e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	-.001	3	-1.799e-5	1	3624.317	2	NC	1
135		11	max	.004	1	.007	2	0	1	1.49e-4	3	NC	3	NC	1
136			min	-.004	3	-.006	3	0	3	-2.382e-5	1	4137.283	2	NC	1
137		12	max	.003	1	.006	2	0	1	1.447e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	0	3	-2.964e-5	1	4800.921	2	NC	1
139		13	max	.003	1	.005	2	0	1	1.404e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-3.547e-5	1	5690.615	2	NC	1
141		14	max	.002	1	.004	2	0	1	1.361e-4	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-4.13e-5	1	6942.053	2	NC	1
143		15	max	.002	1	.003	2	0	1	1.317e-4	3	NC	1	NC	1
144			min	-.002	3	-.003	3	0	3	-4.712e-5	1	8826.601	2	NC	1
145		16	max	.001	1	.003	2	0	1	1.274e-4	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-5.295e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.231e-4	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-5.877e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.188e-4	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-6.46e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.145e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-7.043e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.166e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-5.184e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	2.685e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	1	-4.019e-5	3	NC	1	NC	1
157		3	max	0	9	.002	2	0	3	2.205e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-2.854e-5	3	NC	1	NC	1
159		4	max	0	9	.003	2	0	3	1.724e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-1.688e-5	3	NC	1	NC	1
161		5	max	0	9	.005	2	0	3	1.244e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-5.228e-6	3	NC	1	NC	1
163		6	max	0	9	.006	2	.001	3	7.637e-6	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	0	10	8119.38	2	NC	1
165		7	max	0	9	.007	2	.001	3	1.808e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	0	10	6721.369	2	NC	1
167		8	max	0	9	.008	2	.001	3	2.973e-5	3	NC	3	NC	1
168			min	0	2	-.009	3	-.001	1	-1.971e-6	1	5693.969	2	NC	1
169		9	max	0	9	.009	2	.001	3	4.139e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	-.001	1	-6.775e-6	1	4903.087	2	NC	1
171		10	max	0	9	.011	2	.002	3	5.304e-5	3	NC	3	NC	1
172			min	-.001	2	-.012	3	-.001	1	-1.158e-5	1	4274.315	2	NC	1
173		11	max	0	9	.012	2	.002	3	6.47e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	-.001	1	-1.638e-5	1	3762.836	2	NC	1
175		12	max	0	9	.014	2	.002	3	7.635e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-2.119e-5	1	3339.82	2	NC	1
177		13	max	.001	9	.015	2	.002	3	8.8e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.002	1	-2.599e-5	1	2985.685	2	NC	1
179		14	max	.001	9	.017	2	.002	3	9.966e-5	3	NC	3	NC	1
180			min	-.001	2	-.016	3	-.002	1	-3.08e-5	1	2686.515	2	NC	1
181		15	max	.001	9	.019	2	.002	3	1.113e-4	3	NC	3	NC	1
182			min	-.002	2	-.017	3	-.002	1	-3.56e-5	1	2432.052	2	NC	1
183		16	max	.001	9	.021	2	.002	3	1.23e-4	3	NC	3	NC	1
184			min	-.002	2	-.017	3	-.002	1	-4.04e-5	1	2214.499	2	NC	1
185		17	max	.001	9	.023	2	.002	3	1.346e-4	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-4.521e-5	1	2027.8	2	NC	1
187		18	max	.001	9	.025	2	.001	3	1.463e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	-.002	1	-5.001e-5	1	1867.16	2	NC	1
189		19	max	.002	9	.027	2	.001	3	1.579e-4	3	NC	3	NC	1
190			min	-.002	2	-.019	3	-.002	1	-5.482e-5	1	1728.74	2	NC	1
191	M8	1	max	.007	1	.021	2	.002	1	-6.698e-8	10	NC	1	NC	1
192			min	-.002	3	-.015	3	0	3	-1.257e-4	3	NC	1	NC	1
193		2	max	.006	1	.02	2	.002	1	-6.698e-8	10	NC	1	NC	1
194			min	-.002	3	-.014	3	0	3	-1.257e-4	3	NC	1	NC	1
195		3	max	.006	1	.019	2	.001	1	-6.698e-8	10	NC	1	NC	1
196			min	-.002	3	-.013	3	0	3	-1.257e-4	3	NC	1	NC	1
197		4	max	.006	1	.018	2	.001	1	-6.698e-8	10	NC	1	NC	1
198			min	-.002	3	-.012	3	0	3	-1.257e-4	3	NC	1	NC	1
199		5	max	.005	1	.017	2	.001	1	-6.698e-8	10	NC	1	NC	1
200			min	-.001	3	-.011	3	0	3	-1.257e-4	3	NC	1	NC	1
201		6	max	.005	1	.015	2	.001	1	-6.698e-8	10	NC	1	NC	1
202			min	-.001	3	-.01	3	0	3	-1.257e-4	3	NC	1	NC	1
203		7	max	.005	1	.014	2	0	1	-6.698e-8	10	NC	1	NC	1
204			min	-.001	3	-.01	3	0	3	-1.257e-4	3	NC	1	NC	1
205		8	max	.004	1	.013	2	0	1	-6.698e-8	10	NC	1	NC	1
206			min	-.001	3	-.009	3	0	3	-1.257e-4	3	NC	1	NC	1
207		9	max	.004	1	.012	2	0	1	-6.698e-8	10	NC	1	NC	1
208			min	-.001	3	-.008	3	0	3	-1.257e-4	3	NC	1	NC	1
209		10	max	.003	1	.011	2	0	1	-6.698e-8	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.257e-4	3	NC	1	NC	1
211		11	max	.003	1	.009	2	0	1	-6.698e-8	10	NC	1	NC	1
212			min	0	3	-.006	3	0	3	-1.257e-4	3	NC	1	NC	1
213		12	max	.003	1	.008	2	0	1	-6.698e-8	10	NC	1	NC	1
214			min	0	3	-.006	3	0	3	-1.257e-4	3	NC	1	NC	1
215		13	max	.002	1	.007	2	0	1	-6.698e-8	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.257e-4	3	NC	1	NC	1
217		14	max	.002	1	.006	2	0	1	-6.698e-8	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.257e-4	3	NC	1	NC	1
219		15	max	.002	1	.005	2	0	1	-6.698e-8	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.257e-4	3	NC	1	NC	1
221		16	max	.001	1	.004	2	0	1	-6.698e-8	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.257e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.002	2	0	1	-6.698e-8	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.257e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-6.698e-8	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.257e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-6.698e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.257e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.006	2	0	3	5.631e-4	1	NC	3	NC	1
230			min	-.003	3	-.005	3	-.001	1	-2.429e-4	3	4857.58	2	NC	1
231		2	max	.002	1	.006	2	0	3	5.344e-4	1	NC	3	NC	1
232			min	-.002	3	-.005	3	-.001	1	-2.36e-4	3	5259.326	2	NC	1
233		3	max	.002	1	.005	2	0	3	5.056e-4	1	NC	3	NC	1
234			min	-.002	3	-.005	3	0	1	-2.291e-4	3	5730.134	2	NC	1
235		4	max	.002	1	.005	2	0	3	4.769e-4	1	NC	3	NC	1
236			min	-.002	3	-.004	3	0	1	-2.222e-4	3	6285.595	2	NC	1
237		5	max	.002	1	.004	2	0	3	4.481e-4	1	NC	3	NC	1
238			min	-.002	3	-.004	3	0	1	-2.153e-4	3	6946.187	2	NC	1
239		6	max	.002	1	.004	2	0	3	4.194e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	0	1	-2.084e-4	3	7739.25	2	NC	1
241		7	max	.002	1	.003	2	0	3	3.906e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	0	1	-2.015e-4	3	8701.97	2	NC	1
243		8	max	.002	1	.003	2	0	3	3.619e-4	1	NC	1	NC	1
244			min	-.002	3	-.004	3	0	1	-1.947e-4	3	9886.041	2	NC	1
245		9	max	.001	1	.003	2	0	3	3.332e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-1.878e-4	3	NC	1	NC	1
247		10	max	.001	1	.002	2	0	3	3.044e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-1.809e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	2.757e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-1.74e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	2.469e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.671e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	2.182e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-1.602e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	1.895e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.533e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.607e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.464e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.32e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-1.395e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.032e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-1.326e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	7.448e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.257e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	4.574e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.188e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	5.422e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.153e-5	1	NC	1	NC	1
269		2	max	0	1	0	2	0	1	4.204e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-5.052e-5	1	NC	1	NC	1
271		3	max	0	1	0	2	0	2	2.985e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-7.951e-5	1	NC	1	NC	1
273		4	max	0	1	0	2	0	10	1.767e-5	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-1.085e-4	1	NC	1	NC	1
275		5	max	0	1	0	2	0	10	5.486e-6	3	NC	1	NC	1
276			min	0	10	-.003	3	0	3	-1.375e-4	1	NC	1	NC	1
277		6	max	0	1	0	2	0	10	-4.589e-6	12	NC	1	NC	1
278			min	0	10	-.004	3	-.001	3	-1.665e-4	1	NC	1	NC	1
279		7	max	0	1	0	2	0	10	-6.533e-6	15	NC	1	NC	1





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-.004	3	-.001	3	-1.955e-4	1	NC	1	NC	1
281		8	max	0	1	.001	2	0	10	-7.476e-6	15	NC	1	NC	1
282			min	0	10	-.005	3	-.001	1	-2.245e-4	1	NC	1	NC	1
283		9	max	0	1	.002	2	0	10	-8.419e-6	15	NC	1	NC	1
284			min	0	10	-.005	3	-.002	1	-2.535e-4	1	NC	1	NC	1
285		10	max	0	1	.002	2	0	15	-9.362e-6	15	NC	1	NC	1
286			min	0	10	-.006	3	-.002	1	-2.825e-4	1	NC	1	NC	1
287		11	max	0	1	.002	2	0	15	-1.031e-5	15	NC	1	NC	1
288			min	0	10	-.006	3	-.003	1	-3.115e-4	1	NC	1	NC	1
289		12	max	0	1	.003	2	0	15	-1.125e-5	15	NC	1	NC	1
290			min	0	10	-.007	3	-.004	1	-3.405e-4	1	NC	1	NC	1
291		13	max	0	1	.004	2	0	15	-1.219e-5	15	NC	1	NC	1
292			min	0	10	-.007	3	-.004	1	-3.694e-4	1	NC	1	NC	1
293		14	max	0	1	.005	2	0	15	-1.313e-5	15	NC	1	NC	2
294			min	0	10	-.007	3	-.005	1	-3.984e-4	1	NC	1	9907.618	1
295		15	max	0	1	.005	2	0	15	-1.408e-5	15	NC	3	NC	2
296			min	0	10	-.007	3	-.005	1	-4.274e-4	1	8529.698	2	8868.887	1
297		16	max	0	1	.006	2	0	15	-1.502e-5	15	NC	3	NC	2
298			min	0	10	-.007	3	-.006	1	-4.564e-4	1	7280.819	2	8057.255	1
299		17	max	0	1	.007	2	0	15	-1.596e-5	15	NC	3	NC	2
300			min	0	10	-.007	3	-.006	1	-4.854e-4	1	6302.26	2	7418.512	1
301		18	max	0	1	.008	2	0	15	-1.691e-5	15	NC	3	NC	2
302			min	0	10	-.007	3	-.007	1	-5.144e-4	1	5528.726	2	6915.453	1
303		19	max	0	1	.009	2	0	15	-1.785e-5	15	NC	3	NC	2
304			min	0	10	-.007	3	-.007	1	-5.434e-4	1	4913.022	2	6522.287	1
305	M12	1	max	.002	1	.007	2	.006	1	4.567e-4	1	NC	1	NC	2
306			min	0	3	-.005	3	0	15	1.491e-5	15	NC	1	3228.143	1
307		2	max	.002	1	.007	2	.005	1	4.567e-4	1	NC	1	NC	2
308			min	0	3	-.005	3	0	15	1.491e-5	15	NC	1	3520.456	1
309		3	max	.002	1	.006	2	.005	1	4.567e-4	1	NC	1	NC	2
310			min	0	3	-.005	3	0	15	1.491e-5	15	NC	1	3868.41	1
311		4	max	.002	1	.006	2	.005	1	4.567e-4	1	NC	1	NC	2
312			min	0	3	-.005	3	0	15	1.491e-5	15	NC	1	4286.671	1
313		5	max	.002	1	.006	2	.004	1	4.567e-4	1	NC	1	NC	2
314			min	0	3	-.004	3	0	15	1.491e-5	15	NC	1	4795.218	1
315		6	max	.002	1	.005	2	.004	1	4.567e-4	1	NC	1	NC	2
316			min	0	3	-.004	3	0	15	1.491e-5	15	NC	1	5421.839	1
317		7	max	.002	1	.005	2	.003	1	4.567e-4	1	NC	1	NC	2
318			min	0	3	-.004	3	0	15	1.491e-5	15	NC	1	6206.118	1
319		8	max	.001	1	.004	2	.003	1	4.567e-4	1	NC	1	NC	2
320			min	0	3	-.003	3	0	15	1.491e-5	15	NC	1	7206.019	1
321		9	max	.001	1	.004	2	.002	1	4.567e-4	1	NC	1	NC	2
322			min	0	3	-.003	3	0	15	1.491e-5	15	NC	1	8509.221	1
323		10	max	.001	1	.004	2	.002	1	4.567e-4	1	NC	1	NC	1
324			min	0	3	-.003	3	0	15	1.491e-5	15	NC	1	NC	1
325		11	max	.001	1	.003	2	.002	1	4.567e-4	1	NC	1	NC	1
326			min	0	3	-.002	3	0	15	1.491e-5	15	NC	1	NC	1
327		12	max	0	1	.003	2	.001	1	4.567e-4	1	NC	1	NC	1
328			min	0	3	-.002	3	0	15	1.491e-5	15	NC	1	NC	1
329		13	max	0	1	.002	2	0	1	4.567e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	1.491e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	0	1	4.567e-4	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	1.491e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	4.567e-4	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	1.491e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	4.567e-4	1	NC	1	NC	1
336			min	0	3	0	3	0	15	1.491e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	4.567e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	1.491e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.567e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	1.491e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.567e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.491e-5	15	NC	1	NC	1
343	M1	1	max	.005	3	.023	3	.001	3	9.831e-3	1	NC	1	NC	1
344			min	-.006	2	-.026	1	-.003	1	-9.72e-3	3	NC	1	NC	1
345		2	max	.005	3	.012	3	.001	3	4.704e-3	1	NC	4	NC	1
346			min	-.006	2	-.014	1	-.006	1	-4.794e-3	3	3740.392	1	NC	1
347		3	max	.005	3	.002	3	0	3	4.189e-5	3	NC	4	NC	1
348			min	-.006	2	-.002	1	-.007	1	-3.276e-4	1	1934.801	1	NC	1
349		4	max	.005	3	.008	1	0	3	4.175e-5	3	NC	5	NC	2
350			min	-.006	2	-.006	3	-.008	1	-2.729e-4	1	1368.019	1	8482.605	1
351		5	max	.005	3	.016	1	0	3	4.16e-5	3	NC	5	NC	2
352			min	-.006	2	-.012	3	-.009	1	-2.182e-4	1	1095.479	1	8163.626	1
353		6	max	.005	3	.023	1	0	3	4.146e-5	3	NC	5	NC	2
354			min	-.006	2	-.018	3	-.008	1	-1.636e-4	1	941.359	1	8764.313	1
355		7	max	.005	3	.028	1	0	3	4.132e-5	3	NC	5	NC	1
356			min	-.006	2	-.022	3	-.007	1	-1.089e-4	1	847.978	1	NC	1
357		8	max	.005	3	.032	1	0	3	4.118e-5	3	NC	5	NC	1
358			min	-.006	2	-.024	3	-.006	1	-5.427e-5	1	791.341	1	NC	1
359		9	max	.005	3	.034	1	0	3	4.103e-5	3	NC	5	NC	1
360			min	-.006	2	-.026	3	-.004	1	2.182e-7	15	760.242	1	NC	1
361		10	max	.005	3	.035	1	0	3	5.505e-5	1	NC	5	NC	1
362			min	-.006	2	-.026	3	-.003	1	1.904e-6	15	749.502	1	NC	1
363		11	max	.005	3	.034	1	0	3	1.097e-4	1	NC	5	NC	1
364			min	-.006	2	-.025	3	0	1	3.591e-6	15	757.533	1	NC	1
365		12	max	.005	3	.032	1	0	1	1.644e-4	1	NC	5	NC	1
366			min	-.006	2	-.023	3	0	15	5.277e-6	15	785.683	1	NC	1
367		13	max	.005	3	.028	1	.002	1	2.19e-4	1	NC	5	NC	1
368			min	-.006	2	-.02	3	0	15	6.963e-6	15	838.816	1	NC	1
369		14	max	.005	3	.022	1	.003	1	2.737e-4	1	NC	5	NC	2
370			min	-.006	2	-.016	3	0	15	8.649e-6	15	927.62	1	9059.368	1
371		15	max	.005	3	.015	1	.003	1	3.283e-4	1	NC	5	NC	2
372			min	-.006	2	-.011	3	0	15	1.034e-5	15	1075.04	1	8378.268	1
373		16	max	.005	3	.007	1	.003	1	3.686e-4	1	NC	5	NC	2
374			min	-.006	2	-.005	3	0	15	1.159e-5	15	1335.881	1	8657.321	1
375		17	max	.005	3	.002	3	.002	1	6.736e-5	1	NC	4	NC	1
376			min	-.006	2	-.003	1	0	15	2.544e-6	15	1875.52	1	NC	1
377		18	max	.005	3	.009	3	0	1	5.625e-3	1	NC	4	NC	1
378			min	-.006	2	-.016	1	0	15	-2.361e-3	3	3613.522	1	NC	1
379		19	max	.005	3	.017	3	0	3	1.129e-2	1	NC	1	NC	1
380			min	-.006	2	-.029	1	-.002	1	-4.798e-3	3	NC	1	NC	1
381	M5	1	max	.013	3	.065	3	.001	3	9.232e-7	3	NC	1	NC	1
382			min	-.018	2	-.075	1	-.003	1	0	15	NC	1	NC	1
383		2	max	.013	3	.035	3	.002	3	5.119e-5	3	NC	5	NC	1
384			min	-.019	2	-.04	1	-.003	1	-7.4e-5	1	1325.898	1	NC	1
385		3	max	.013	3	.008	3	.003	3	1.005e-4	3	NC	5	NC	1
386			min	-.019	2	-.007	1	-.003	1	-1.466e-4	1	682.256	1	NC	1
387		4	max	.013	3	.021	1	.003	3	9.924e-5	3	NC	5	NC	1
388			min	-.019	2	-.015	3	-.003	1	-1.389e-4	1	481.383	1	NC	1
389		5	max	.013	3	.045	1	.003	3	9.799e-5	3	NC	5	NC	1
390			min	-.019	2	-.034	3	-.003	1	-1.312e-4	1	384.812	1	NC	1
391		6	max	.013	3	.065	1	.004	3	9.674e-5	3	NC	5	NC	1
392			min	-.019	2	-.048	3	-.003	1	-1.235e-4	1	330.161	1	NC	1
393		7	max	.013	3	.08	1	.004	3	9.548e-5	3	NC	15	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.019	2	-.059	3	-.003	1	-1.158e-4	1	296.983	1	NC	1
395		8	max	.013	3	.091	1	.004	3	9.423e-5	3	NC	15	NC	1
396			min	-.019	2	-.067	3	-.003	1	-1.081e-4	1	276.779	1	NC	1
397		9	max	.013	3	.098	1	.003	3	9.298e-5	3	NC	15	NC	1
398			min	-.019	2	-.071	3	-.003	1	-1.004e-4	1	265.573	1	NC	1
399		10	max	.013	3	.1	1	.003	3	9.172e-5	3	NC	15	NC	1
400			min	-.019	2	-.071	3	-.002	1	-9.275e-5	1	261.523	1	NC	1
401		11	max	.013	3	.098	1	.003	3	9.047e-5	3	NC	15	NC	1
402			min	-.019	2	-.068	3	-.002	1	-8.506e-5	1	264.052	1	NC	1
403		12	max	.013	3	.091	1	.003	3	8.922e-5	3	NC	15	NC	1
404			min	-.019	2	-.063	3	-.002	1	-7.738e-5	1	273.616	1	NC	1
405		13	max	.013	3	.08	1	.002	3	8.796e-5	3	NC	15	NC	1
406			min	-.019	2	-.054	3	-.002	1	-6.969e-5	1	291.902	1	NC	1
407		14	max	.013	3	.064	1	.002	3	8.671e-5	3	NC	5	NC	1
408			min	-.019	2	-.043	3	-.002	1	-6.2e-5	1	322.639	1	NC	1
409		15	max	.013	3	.044	1	.002	3	8.546e-5	3	NC	5	NC	1
410			min	-.019	2	-.029	3	-.002	1	-5.432e-5	1	373.859	1	NC	1
411		16	max	.014	3	.019	1	.001	3	8.236e-5	3	NC	5	NC	1
412			min	-.019	2	-.013	3	-.002	1	-5.016e-5	1	464.836	1	NC	1
413		17	max	.014	3	.005	3	0	3	3.533e-5	3	NC	5	NC	1
414			min	-.019	2	-.011	1	-.002	1	-1.3e-4	1	654.46	1	NC	1
415		18	max	.014	3	.026	3	0	3	1.732e-5	3	NC	5	NC	1
416			min	-.019	2	-.046	1	-.002	1	-6.635e-5	1	1268.013	1	NC	1
417		19	max	.014	3	.047	3	0	3	0	5	NC	1	NC	1
418			min	-.019	2	-.083	1	-.002	1	-1.437e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.023	3	.001	3	9.722e-3	3	NC	1	NC	1
420			min	-.006	2	-.026	1	-.004	1	-9.831e-3	1	NC	1	NC	1
421		2	max	.005	3	.012	3	0	3	4.827e-3	3	NC	4	NC	1
422			min	-.006	2	-.014	1	0	1	-4.857e-3	1	3741.736	1	NC	1
423		3	max	.005	3	.002	3	.001	1	2.532e-5	2	NC	4	NC	1
424			min	-.006	2	-.002	1	0	3	7.909e-7	15	1935.521	1	NC	1
425		4	max	.005	3	.008	1	.002	1	1.337e-5	3	NC	5	NC	1
426			min	-.006	2	-.006	3	0	3	-1.668e-5	1	1368.536	1	NC	1
427		5	max	.005	3	.016	1	.002	1	5.088e-6	10	NC	5	NC	1
428			min	-.006	2	-.012	3	-.001	3	-5.762e-5	1	1095.884	1	NC	1
429		6	max	.005	3	.023	1	.002	1	2.029e-6	10	NC	5	NC	1
430			min	-.006	2	-.018	3	-.002	3	-9.856e-5	1	941.695	1	NC	1
431		7	max	.005	3	.028	1	.001	1	-1.029e-6	10	NC	5	NC	1
432			min	-.006	2	-.022	3	-.002	3	-1.395e-4	1	848.267	1	NC	1
433		8	max	.005	3	.032	1	0	2	-4.088e-6	10	NC	5	NC	1
434			min	-.006	2	-.024	3	-.002	3	-1.804e-4	1	791.598	1	NC	1
435		9	max	.005	3	.034	1	0	10	-7.147e-6	10	NC	5	NC	1
436			min	-.006	2	-.026	3	-.002	3	-2.214e-4	1	760.475	1	NC	1
437		10	max	.005	3	.035	1	0	10	-8.539e-6	15	NC	5	NC	1
438			min	-.006	2	-.026	3	-.002	3	-2.623e-4	1	749.718	1	NC	1
439		11	max	.005	3	.034	1	0	10	-9.871e-6	15	NC	5	NC	1
440			min	-.006	2	-.025	3	-.004	1	-3.033e-4	1	757.737	1	NC	1
441		12	max	.005	3	.032	1	0	15	-1.12e-5	15	NC	5	NC	1
442			min	-.006	2	-.023	3	-.005	1	-3.442e-4	1	785.88	1	NC	1
443		13	max	.005	3	.028	1	0	15	-1.254e-5	15	NC	5	NC	2
444			min	-.006	2	-.02	3	-.006	1	-3.851e-4	1	839.011	1	9443.254	1
445		14	max	.005	3	.022	1	0	15	-1.387e-5	15	NC	5	NC	2
446			min	-.006	2	-.016	3	-.007	1	-4.261e-4	1	927.818	1	8202.277	1
447		15	max	.005	3	.015	1	0	15	-1.52e-5	15	NC	5	NC	2
448			min	-.006	2	-.011	3	-.007	1	-4.67e-4	1	1075.25	1	7817.631	1
449		16	max	.005	3	.007	1	0	15	-1.623e-5	15	NC	5	NC	2
450			min	-.006	2	-.005	3	-.007	1	-4.995e-4	1	1336.121	1	8236.635	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	15	-9.911e-6	15	NC	4	NC	1
452			min	-.006	2	-.003	1	-.006	1	-3.308e-4	1	1875.834	1	NC	1
453		18	max	.005	3	.009	3	0	15	2.367e-3	3	NC	4	NC	1
454			min	-.006	2	-.016	1	-.004	1	-5.76e-3	1	3614.101	1	NC	1
455		19	max	.005	3	.017	3	0	3	4.797e-3	3	NC	1	NC	1
456			min	-.006	2	-.029	1	-.001	1	-1.129e-2	1	NC	1	NC	1
457	M13	1	max	.004	1	.023	3	.005	3	4.023e-3	3	NC	1	NC	1
458			min	-.001	3	-.026	1	-.006	2	-4.724e-3	1	NC	1	NC	1
459		2	max	.004	1	.101	3	.012	1	4.813e-3	3	NC	4	NC	2
460			min	-.001	3	-.105	1	-.002	10	-5.661e-3	1	1895.879	1	8914.348	1
461		3	max	.004	1	.165	3	.034	1	5.603e-3	3	NC	5	NC	2
462			min	-.001	3	-.171	1	-.001	10	-6.597e-3	1	1037.817	1	3924.323	1
463		4	max	.003	1	.207	3	.051	1	6.393e-3	3	NC	5	NC	3
464			min	-.001	3	-.213	1	0	10	-7.534e-3	1	802.26	1	2706.176	1
465		5	max	.003	1	.221	3	.058	1	7.183e-3	3	NC	5	NC	3
466			min	-.001	3	-.228	1	0	10	-8.471e-3	1	742.605	1	2383.521	1
467		6	max	.003	1	.208	3	.054	1	7.974e-3	3	NC	5	NC	3
468			min	-.001	3	-.216	1	-.002	10	-9.407e-3	1	789.522	1	2565.398	1
469		7	max	.003	1	.174	3	.038	1	8.764e-3	3	NC	5	NC	2
470			min	-.001	3	-.182	1	-.004	10	-1.034e-2	1	960.375	1	3485.955	1
471		8	max	.003	1	.127	3	.016	1	9.554e-3	3	NC	5	NC	2
472			min	-.001	3	-.137	1	-.007	10	-1.128e-2	1	1355.556	1	7195.988	1
473		9	max	.003	1	.085	3	.012	3	1.034e-2	3	NC	4	NC	1
474			min	-.001	3	-.094	1	-.014	2	-1.222e-2	1	2196.713	1	NC	1
475		10	max	.003	1	.065	3	.013	3	1.113e-2	3	NC	4	NC	1
476			min	-.001	3	-.075	1	-.018	2	-1.315e-2	1	3073.594	1	NC	1
477		11	max	.003	1	.085	3	.015	3	1.034e-2	3	NC	4	NC	1
478			min	-.001	3	-.094	1	-.014	2	-1.222e-2	1	2196.714	1	NC	1
479		12	max	.003	1	.128	3	.017	1	9.555e-3	3	NC	5	NC	2
480			min	-.001	3	-.137	1	-.007	10	-1.128e-2	1	1355.556	1	7098.359	1
481		13	max	.003	1	.174	3	.039	1	8.765e-3	3	NC	5	NC	2
482			min	-.001	3	-.182	1	-.004	10	-1.034e-2	1	960.375	1	3468.035	1
483		14	max	.003	1	.208	3	.054	1	7.975e-3	3	NC	5	NC	3
484			min	-.001	3	-.216	1	-.002	10	-9.408e-3	1	789.523	1	2561.694	1
485		15	max	.003	1	.221	3	.058	1	7.186e-3	3	NC	5	NC	3
486			min	-.001	3	-.228	1	0	10	-8.471e-3	1	742.605	1	2386.478	1
487		16	max	.003	1	.207	3	.051	1	6.396e-3	3	NC	5	NC	3
488			min	-.001	3	-.213	1	0	10	-7.535e-3	1	802.261	1	2716.785	1
489		17	max	.003	1	.165	3	.033	1	5.606e-3	3	NC	5	NC	2
490			min	-.001	3	-.171	1	-.001	10	-6.598e-3	1	1037.818	1	3953.501	1
491		18	max	.003	1	.101	3	.012	1	4.817e-3	3	NC	4	NC	2
492			min	-.001	3	-.105	1	-.002	10	-5.662e-3	1	1895.88	1	9035.819	1
493		19	max	.003	1	.023	3	.005	3	4.027e-3	3	NC	1	NC	1
494			min	-.001	3	-.026	1	-.006	2	-4.725e-3	1	NC	1	NC	1
495	M16	1	max	.001	1	.017	3	.005	3	4.998e-3	1	NC	1	NC	1
496			min	0	3	-.029	1	-.006	2	-3.044e-3	3	NC	1	NC	1
497		2	max	.001	1	.056	3	.012	1	6.009e-3	1	NC	4	NC	2
498			min	0	3	-.12	1	-.002	10	-3.608e-3	3	1650.421	1	8939.903	1
499		3	max	.001	1	.088	3	.033	1	7.02e-3	1	NC	5	NC	2
500			min	0	3	-.195	1	-.001	10	-4.172e-3	3	903.679	1	3935.255	1
501		4	max	.001	1	.109	3	.05	1	8.031e-3	1	NC	5	NC	3
502			min	0	3	-.243	1	0	10	-4.736e-3	3	698.885	1	2714.569	1
503		5	max	.001	1	.117	3	.058	1	9.042e-3	1	NC	5	NC	3
504			min	0	3	-.26	1	0	10	-5.3e-3	3	647.402	1	2392.676	1
505		6	max	.002	1	.112	3	.053	1	1.005e-2	1	NC	5	NC	3
506			min	0	3	-.246	1	-.002	10	-5.864e-3	3	689.173	1	2579.141	1
507		7	max	.002	1	.097	3	.038	1	1.106e-2	1	NC	5	NC	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-207	1	-.004	10	-6.428e-3	3	840.198	1	3517.079	1
509	8	max	.002	1	.076	3	.016	3	1.207e-2	1	NC	5	NC	2
510		min	0	3	-.155	1	-.007	10	-6.992e-3	3	1191.165	1	7353.176	1
511	9	max	.002	1	.056	3	.015	3	1.309e-2	1	NC	4	NC	1
512		min	0	3	-.106	1	-.015	2	-7.556e-3	3	1947.631	1	NC	1
513	10	max	.002	1	.047	3	.014	3	1.41e-2	1	NC	4	NC	1
514		min	0	3	-.083	1	-.019	2	-8.12e-3	3	2750.349	1	NC	1
515	11	max	.002	1	.056	3	.013	3	1.309e-2	1	NC	4	NC	1
516		min	0	3	-.106	1	-.015	2	-7.555e-3	3	1947.631	1	NC	1
517	12	max	.002	1	.076	3	.016	1	1.208e-2	1	NC	5	NC	2
518		min	0	3	-.155	1	-.007	10	-6.991e-3	3	1191.165	1	7296.515	1
519	13	max	.002	1	.097	3	.038	1	1.106e-2	1	NC	5	NC	2
520		min	0	3	-207	1	-.004	10	-6.427e-3	3	840.198	1	3512.349	1
521	14	max	.002	1	.112	3	.053	1	1.005e-2	1	NC	5	NC	3
522		min	0	3	-.246	1	-.002	10	-5.862e-3	3	689.173	1	2583.524	1
523	15	max	.002	1	.117	3	.058	1	9.042e-3	1	NC	5	NC	3
524		min	0	3	-.26	1	0	10	-5.298e-3	3	647.402	1	2402.684	1
525	16	max	.002	1	.109	3	.05	1	8.032e-3	1	NC	5	NC	3
526		min	0	3	-.243	1	0	10	-4.734e-3	3	698.885	1	2733.429	1
527	17	max	.002	1	.088	3	.033	1	7.021e-3	1	NC	5	NC	2
528		min	0	3	-.195	1	-.001	10	-4.169e-3	3	903.68	1	3978.074	1
529	18	max	.002	1	.056	3	.012	1	6.01e-3	1	NC	4	NC	2
530		min	0	3	-.12	1	-.002	10	-3.605e-3	3	1650.422	1	9102.346	1
531	19	max	.002	1	.017	3	.005	3	4.999e-3	1	NC	1	NC	1
532		min	0	3	-.029	1	-.006	2	-3.041e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	2.998e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.977e-5	2	NC	1	NC	1
535	2	max	0	3	-.002	15	0	1	7.746e-4	3	NC	1	NC	1
536		min	0	2	-.009	4	0	3	-6.496e-4	1	8669.266	4	NC	1
537	3	max	0	3	-.004	15	.003	1	1.249e-3	3	NC	5	NC	1
538		min	0	2	-.018	4	-.003	3	-1.249e-3	1	4411.494	4	NC	1
539	4	max	0	3	-.006	15	.007	1	1.724e-3	3	NC	15	NC	4
540		min	0	2	-.027	4	-.006	3	-1.848e-3	1	3026.542	4	8406.306	1
541	5	max	0	3	-.008	15	.011	1	2.199e-3	3	NC	15	NC	4
542		min	0	2	-.034	4	-.009	3	-2.447e-3	1	2361.642	4	5450.727	1
543	6	max	0	3	-.01	15	.016	1	2.674e-3	3	8455.413	15	NC	4
544		min	0	2	-.041	4	-.014	3	-3.047e-3	1	1987.571	4	3936.005	1
545	7	max	0	3	-.011	15	.021	1	3.149e-3	3	7498.426	15	NC	4
546		min	0	2	-.046	4	-.018	3	-3.646e-3	1	1762.617	4	3058.369	1
547	8	max	0	3	-.012	15	.026	1	3.623e-3	3	6924.09	15	NC	4
548		min	0	2	-.05	4	-.022	3	-4.245e-3	1	1627.611	4	2510.122	1
549	9	max	0	3	-.012	15	.03	1	4.098e-3	3	6614.948	15	NC	4
550		min	0	2	-.052	4	-.026	3	-4.844e-3	1	1554.942	4	2152.781	1
551	10	max	0	3	-.012	15	.034	1	4.573e-3	3	6517.151	15	NC	4
552		min	0	2	-.053	4	-.029	3	-5.444e-3	1	1531.954	4	1917.321	1
553	11	max	0	3	-.012	15	.036	1	5.048e-3	3	6614.948	15	NC	5
554		min	0	2	-.053	4	-.031	3	-6.043e-3	1	1554.942	4	1767.631	1
555	12	max	0	3	-.012	15	.038	1	5.523e-3	3	6924.09	15	NC	5
556		min	-.001	2	-.05	4	-.032	3	-6.642e-3	1	1627.611	4	1685.706	1
557	13	max	0	3	-.011	15	.037	1	5.997e-3	3	7498.426	15	NC	5
558		min	-.001	2	-.047	4	-.032	3	-7.241e-3	1	1762.617	4	1665.977	1
559	14	max	0	3	-.01	15	.035	1	6.472e-3	3	8455.413	15	NC	5
560		min	-.001	2	-.041	4	-.03	3	-7.841e-3	1	1987.571	4	1715.24	1
561	15	max	0	3	-.008	15	.03	1	6.947e-3	3	NC	15	NC	4
562		min	-.001	2	-.035	4	-.026	3	-8.44e-3	1	2361.642	4	1859.741	1
563	16	max	0	3	-.006	15	.023	1	7.422e-3	3	NC	15	NC	4
564		min	-.001	2	-.028	4	-.02	3	-9.039e-3	1	3026.542	4	2171.297	1

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	-.004	15	.013	1	7.897e-3	3	NC	5	NC	4
566			min	-.001	2	-.019	4	-.011	3	-9.638e-3	1	4411.494	4	2875.647	1
567		18	max	0	3	-.001	12	.002	9	8.371e-3	3	NC	1	NC	4
568			min	-.002	2	-.011	1	-.005	2	-1.024e-2	1	8669.266	4	5115.22	1
569		19	max	0	3	.005	3	.015	3	8.846e-3	3	NC	1	NC	1
570			min	-.002	2	-.004	1	-.019	2	-1.084e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	3	.005	3	3.077e-3	3	NC	1	NC	1
572			min	0	3	-.001	1	-.007	2	-3.592e-3	1	NC	1	NC	1
573		2	max	0	10	-.002	15	.003	1	2.936e-3	3	NC	1	NC	1
574			min	0	3	-.01	4	0	10	-3.415e-3	1	8669.266	4	NC	1
575		3	max	0	10	-.004	15	.009	1	2.794e-3	3	NC	5	NC	4
576			min	0	3	-.019	4	-.005	3	-3.237e-3	1	4411.494	4	6011.334	1
577		4	max	0	10	-.006	15	.013	1	2.653e-3	3	NC	15	NC	4
578			min	0	3	-.027	4	-.008	3	-3.06e-3	1	3026.542	4	4561.138	1
579		5	max	0	10	-.008	15	.017	1	2.512e-3	3	NC	15	NC	4
580			min	0	3	-.035	4	-.01	3	-2.882e-3	1	2361.642	4	3928.408	1
581		6	max	0	10	-.01	15	.018	1	2.371e-3	3	8455.413	15	NC	4
582			min	0	3	-.041	4	-.012	3	-2.704e-3	1	1987.571	4	3646.328	1
583		7	max	0	10	-.011	15	.019	1	2.229e-3	3	7498.426	15	NC	4
584			min	0	3	-.046	4	-.012	3	-2.527e-3	1	1762.617	4	3567.865	1
585		8	max	0	10	-.012	15	.019	1	2.088e-3	3	6924.09	15	NC	4
586			min	0	3	-.05	4	-.012	3	-2.349e-3	1	1627.611	4	3641.555	1
587		9	max	0	10	-.012	15	.018	1	1.947e-3	3	6614.948	15	NC	4
588			min	0	3	-.052	4	-.012	3	-2.171e-3	1	1554.942	4	3858.141	1
589		10	max	0	10	-.012	15	.016	1	1.806e-3	3	6517.151	15	NC	4
590			min	0	3	-.053	4	-.011	3	-1.994e-3	1	1531.954	4	4237.453	1
591		11	max	0	10	-.012	15	.014	1	1.664e-3	3	6614.948	15	NC	4
592			min	0	3	-.052	4	-.009	3	-1.816e-3	1	1554.942	4	4831.792	1
593		12	max	0	10	-.012	15	.012	1	1.523e-3	3	6924.09	15	NC	4
594			min	0	3	-.05	4	-.008	3	-1.639e-3	1	1627.611	4	5744.949	1
595		13	max	0	10	-.011	15	.01	1	1.382e-3	3	7498.426	15	NC	2
596			min	0	3	-.046	4	-.006	3	-1.461e-3	1	1762.617	4	7180.832	1
597		14	max	0	10	-.01	15	.007	1	1.24e-3	3	8455.413	15	NC	2
598			min	0	3	-.041	4	-.004	3	-1.283e-3	1	1987.571	4	9569.176	1
599		15	max	0	10	-.008	15	.005	1	1.099e-3	3	NC	15	NC	1
600			min	0	3	-.034	4	-.003	3	-1.106e-3	1	2361.642	4	NC	1
601		16	max	0	10	-.006	15	.003	1	9.579e-4	3	NC	15	NC	1
602			min	0	3	-.027	4	-.002	3	-9.28e-4	1	3026.542	4	NC	1
603		17	max	0	10	-.004	15	.001	1	8.166e-4	3	NC	5	NC	1
604			min	0	3	-.018	4	0	3	-7.622e-4	2	4411.494	4	NC	1
605		18	max	0	10	-.002	15	0	4	6.754e-4	3	NC	1	NC	1
606			min	0	3	-.009	4	0	2	-6.056e-4	2	8669.266	4	NC	1
607		19	max	0	1	0	1	0	1	5.341e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-4.49e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.