



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.07	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	75 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	-1.556 k-ft
M_z =	-0.008 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	57%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

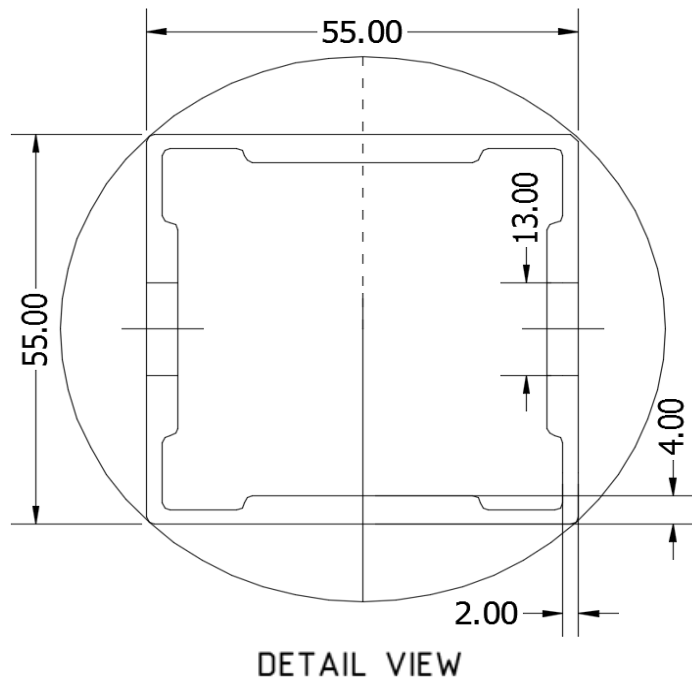
Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.165 k-ft
M_z =	0.000 k-ft
P_n =	1.927 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	96%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

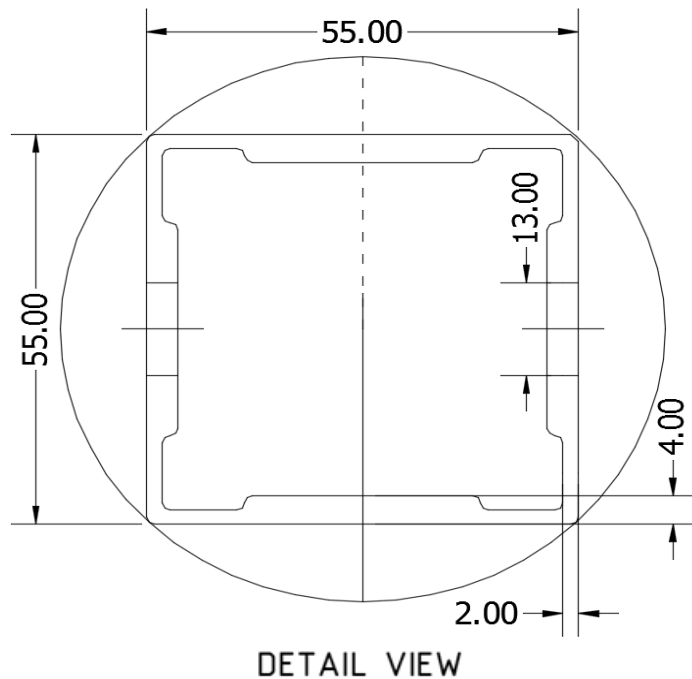
Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.426 k-ft
P_n =	0.348 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	32%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

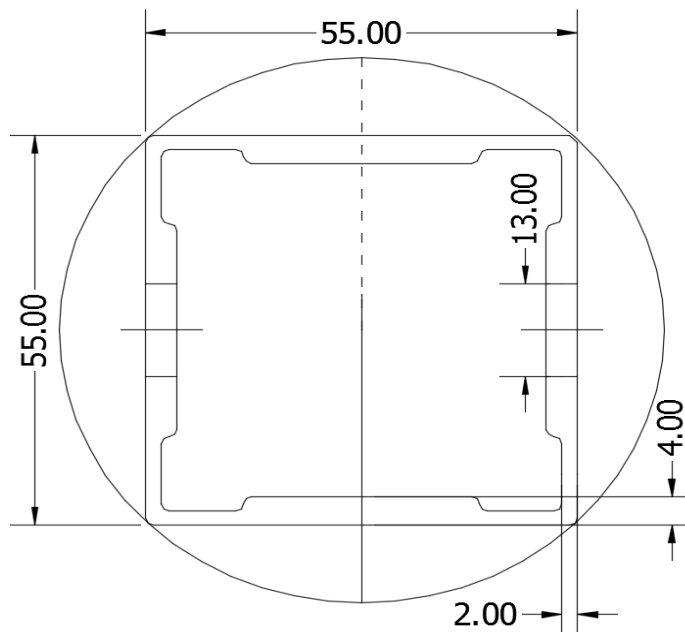
Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.012 k-ft
M_z =	0.000 k-ft
P_n =	2.731 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	46%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.012 k-ft
M_z =	0.000 k-ft
P_n =	3.187 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	37%



5. FOUNDATION DESIGN CALCULATIONS

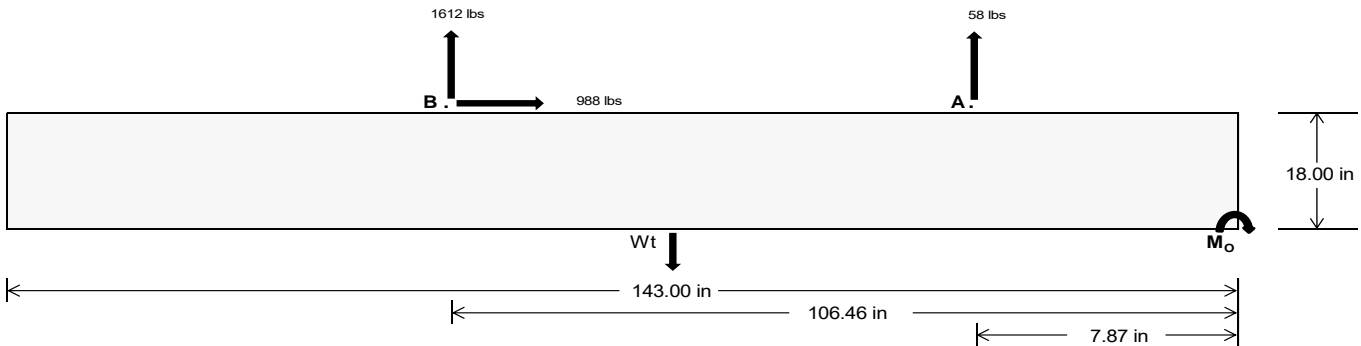
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		266.96	6994.15 k
Compressive Load =		3110.08	4995.82 k
Lateral Load =		293.97	4281.29 k
Moment (Weak Axis) =		0.56	0.19 k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 189808.4$ in-lbs
Resisting Force Required = 2654.66 lbs
S.F. = 1.67
Weight Required = 4424.44 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Sliding

Force = 988.38 lbs
Friction = 0.4
Weight Required = 2470.95 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 988.38 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$ 35 in 36 in 37 in 38 in
7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	878 lbs	878 lbs	878 lbs	878 lbs	1282 lbs	1282 lbs	1282 lbs	1282 lbs	1525 lbs	1525 lbs	1525 lbs	1525 lbs	-116 lbs	-116 lbs	-116 lbs	-116 lbs
F_B	844 lbs	844 lbs	844 lbs	844 lbs	2157 lbs	2157 lbs	2157 lbs	2157 lbs	2159 lbs	2159 lbs	2159 lbs	2159 lbs	-3223 lbs	-3223 lbs	-3223 lbs	-3223 lbs
F_V	97 lbs	97 lbs	97 lbs	97 lbs	1767 lbs	1767 lbs	1767 lbs	1767 lbs	1388 lbs	1388 lbs	1388 lbs	1388 lbs	-1977 lbs	-1977 lbs	-1977 lbs	-1977 lbs
P_{total}	9281 lbs	9497 lbs	9713 lbs	9929 lbs	10999 lbs	11215 lbs	11431 lbs	11647 lbs	11244 lbs	11460 lbs	11676 lbs	11892 lbs	1197 lbs	1326 lbs	1456 lbs	1586 lbs
M	2343 lbs-ft	2343 lbs-ft	2343 lbs-ft	2343 lbs-ft	3167 lbs-ft	3167 lbs-ft	3167 lbs-ft	3167 lbs-ft	3879 lbs-ft	3879 lbs-ft	3879 lbs-ft	3879 lbs-ft	5809 lbs-ft	5809 lbs-ft	5809 lbs-ft	5809 lbs-ft
e	0.25 ft	0.25 ft	0.24 ft	0.24 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.35 ft	0.34 ft	0.33 ft	0.33 ft	4.85 ft	4.38 ft	3.99 ft	3.66 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	233.1 psf	232.7 psf	232.3 psf	231.9 psf	270.6 psf	269.1 psf	267.7 psf	266.4 psf	267.3 psf	265.9 psf	264.6 psf	263.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	301.0 psf	298.7 psf	296.5 psf	294.4 psf	362.3 psf	358.3 psf	354.5 psf	350.9 psf	379.7 psf	375.2 psf	370.9 psf	366.9 psf	247.8 psf	186.7 psf	159.9 psf	145.5 psf

Maximum Bearing Pressure = 380 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

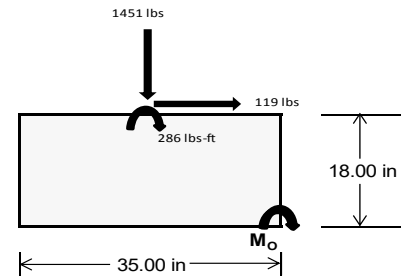
Overturning Check

$M_o = 1651.4 \text{ ft-lbs}$
 Resisting Force Required = 1132.38 lbs
 S.F. = 1.67
 Weight Required = 1887.29 lbs
 Minimum Width = 35 in
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	276 lbs	462 lbs	137 lbs	622 lbs	1451 lbs	516 lbs	129 lbs	135 lbs	-9 lbs
F_v	165 lbs	161 lbs	168 lbs	122 lbs	119 lbs	129 lbs	166 lbs	162 lbs	167 lbs
P_{total}	9635 lbs	9820 lbs	9496 lbs	9531 lbs	10360 lbs	9425 lbs	2866 lbs	2872 lbs	2728 lbs
M	629 lbs-ft	616 lbs-ft	637 lbs-ft	468 lbs-ft	465 lbs-ft	492 lbs-ft	629 lbs-ft	615 lbs-ft	631 lbs-ft
e	0.07 ft	0.06 ft	0.07 ft	0.05 ft	0.04 ft	0.05 ft	0.22 ft	0.21 ft	0.23 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	240.0 psf	246.1 psf	235.5 psf	246.5 psf	270.6 psf	242.1 psf	45.3 psf	46.2 psf	41.1 psf
f_{max}	314.4 psf	319.0 psf	310.9 psf	301.9 psf	325.6 psf	300.3 psf	119.7 psf	119.0 psf	115.9 psf



Maximum Bearing Pressure = 326 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	1.163 k
Allowable Uplift =	1.214 k
Utilization =	<u>96%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.609 k
Allowable Uplift =	4.357 k
Utilization =	<u>60%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.392 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>32%</u>

Rear Strut

Maximum Axial Load =	4.714 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>64%</u>

Diagonal Strut

Maximum Axial Load =	2.888 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	60.93 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.219 in
	<u>0.527 ≤ 1.219, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 75 \text{ in}$$

$$J = 0.432$$

$$207.485$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75$$

$$J = 0.432$$

$$131.948$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 160mph 30psf 6.25ft 7-10Rade 19





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-3.28	10	612.714	10	148.229	1	.011	3	.19	4	1.375	2
134		min	-42.288	1	-1082.725	2	-104.704	14	-.003	1	-.002	3	-.772	3
135	11	max	-2.795	15	891.025	2	-3.014	12	.011	3	.129	4	.69	2
136		min	-42.288	1	-462.949	3	-117.464	1	-.003	1	-.006	3	-.417	3
137	12	max	-3.28	10	699.325	2	-1.565	12	.011	3	.075	4	.138	2
138		min	-42.288	1	-365.98	3	-86.699	1	-.003	1	-.008	3	-.129	3
139	13	max	-3.28	10	507.625	2	.225	3	.011	3	.037	5	.091	3
140		min	-42.288	1	-269.011	3	-64.416	4	-.003	1	-.046	1	-.281	2
141	14	max	-3.28	10	315.925	2	2.399	3	.011	3	.003	5	.244	3
142		min	-42.288	1	-172.042	3	-55.094	4	-.003	1	-.074	1	-.567	2
143	15	max	-3.28	10	124.225	2	5.594	1	.011	3	-.003	12	.33	3
144		min	-47.777	4	-75.073	3	-46.961	5	-.003	1	-.081	1	-.72	2
145	16	max	-3.28	10	21.897	3	36.359	1	.011	3	-.001	12	.349	3
146		min	-55.972	4	-67.475	2	-44.755	5	-.003	1	-.078	4	-.74	2
147	17	max	-3.28	10	118.866	3	67.124	1	.011	3	.004	3	.3	3
148		min	-64.166	4	-259.175	2	-42.548	5	-.003	1	-.1	4	-.626	2
149	18	max	-3.28	10	215.835	3	97.888	1	.011	3	.027	1	.184	3
150		min	-72.361	4	-450.875	2	-40.342	5	-.003	1	-.122	5	-.38	2
151	19	max	-3.28	10	312.804	3	128.653	1	.011	3	.105	1	0	2
152		min	-80.556	4	-642.574	2	-38.136	5	-.003	1	-.149	5	0	5
153	M2	1	max	1103.186	2	2.063	.203	1	0	3	0	3	0	1
154		min	-1581.815	3	.501	15	-18.472	4	0	4	0	2	0	1
155	2	max	1103.715	2	1.992	4	.203	1	0	3	0	1	0	15
156		min	-1581.418	3	.484	15	-18.933	4	0	4	-.007	4	0	4
157	3	max	1104.244	2	1.921	4	.203	1	0	3	0	1	0	15
158		min	-1581.021	3	.468	15	-19.394	4	0	4	-.014	4	-.001	4
159	4	max	1104.774	2	1.85	4	.203	1	0	3	0	1	0	15
160		min	-1580.624	3	.451	15	-19.855	4	0	4	-.021	4	-.002	4
161	5	max	1105.303	2	1.779	4	.203	1	0	3	0	1	0	15
162		min	-1580.227	3	.434	15	-20.317	4	0	4	-.028	4	-.003	4
163	6	max	1105.832	2	1.708	4	.203	1	0	3	0	1	0	15
164		min	-1579.83	3	.418	15	-20.778	4	0	4	-.035	4	-.003	4
165	7	max	1106.362	2	1.637	4	.203	1	0	3	0	1	0	15
166		min	-1579.433	3	.401	15	-21.239	4	0	4	-.043	4	-.004	4
167	8	max	1106.891	2	1.566	4	.203	1	0	3	0	1	-.001	15
168		min	-1579.036	3	.384	15	-21.7	4	0	4	-.05	4	-.005	4
169	9	max	1107.42	2	1.495	4	.203	1	0	3	0	1	-.001	15
170		min	-1578.639	3	.367	15	-22.161	4	0	4	-.058	4	-.005	4
171	10	max	1107.949	2	1.424	4	.203	1	0	3	0	1	-.001	15
172		min	-1578.242	3	.346	12	-22.623	4	0	4	-.066	4	-.006	4
173	11	max	1108.479	2	1.352	4	.203	1	0	3	0	1	-.001	15
174		min	-1577.845	3	.318	12	-23.084	4	0	4	-.075	4	-.006	4
175	12	max	1109.008	2	1.281	4	.203	1	0	3	0	1	-.002	15
176		min	-1577.448	3	.29	12	-23.545	4	0	4	-.083	4	-.007	4
177	13	max	1109.537	2	1.21	4	.203	1	0	3	0	1	-.002	15
178		min	-1577.052	3	.262	12	-24.006	4	0	4	-.091	4	-.007	4
179	14	max	1110.067	2	1.139	4	.203	1	0	3	0	1	-.002	15
180		min	-1576.655	3	.235	12	-24.467	4	0	4	-.1	4	-.007	4
181	15	max	1110.596	2	1.068	4	.203	1	0	3	.001	1	-.002	15
182		min	-1576.258	3	.207	12	-24.929	4	0	4	-.109	4	-.008	4
183	16	max	1111.125	2	.997	4	.203	1	0	3	.001	1	-.002	15
184		min	-1575.861	3	.179	12	-25.39	4	0	4	-.118	4	-.008	4
185	17	max	1111.654	2	.93	2	.203	1	0	3	.001	1	-.002	15
186		min	-1575.464	3	.152	12	-25.851	4	0	4	-.127	4	-.009	4
187	18	max	1112.184	2	.875	2	.203	1	0	3	.001	1	-.002	15
188		min	-1575.067	3	.124	12	-26.312	4	0	4	-.137	4	-.009	4
189	19	max	1112.713	2	.82	2	.203	1	0	3	.001	1	-.002	12



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1574.67	3	.096	12	-26.774	4	0	4	-.146	4	-.009	4
191	M3	1	max	897.74	2	8.905	4	.956	4	0	10	0	1	.009	4
192			min	-1021.088	3	2.106	15	.014	10	0	4	-.016	4	.002	12
193		2	max	897.569	2	8.036	4	1.561	4	0	10	0	1	.005	2
194			min	-1021.215	3	1.901	15	.014	10	0	4	-.015	4	0	12
195		3	max	897.399	2	7.167	4	2.167	4	0	10	0	1	.002	2
196			min	-1021.343	3	1.697	15	.014	10	0	4	-.014	4	-.001	3
197		4	max	897.229	2	6.298	4	2.772	4	0	10	0	1	0	2
198			min	-1021.471	3	1.493	15	.014	10	0	4	-.013	5	-.003	3
199		5	max	897.058	2	5.43	4	3.377	4	0	10	0	1	0	15
200			min	-1021.599	3	1.289	15	.014	10	0	4	-.012	5	-.004	6
201		6	max	896.888	2	4.561	4	3.982	4	0	10	0	1	-.001	15
202			min	-1021.726	3	1.084	15	.014	10	0	4	-.01	5	-.007	6
203		7	max	896.717	2	3.692	4	4.587	4	0	10	0	1	-.002	15
204			min	-1021.854	3	.88	15	.014	10	0	4	-.008	5	-.009	6
205		8	max	896.547	2	2.823	4	5.192	4	0	10	0	1	-.002	15
206			min	-1021.982	3	.676	15	.014	10	0	4	-.006	5	-.01	6
207		9	max	896.377	2	1.954	4	5.797	4	0	10	0	1	-.003	15
208			min	-1022.11	3	.472	15	.014	10	0	4	-.003	5	-.011	6
209		10	max	896.206	2	1.085	4	6.402	4	0	10	0	1	-.003	15
210			min	-1022.237	3	.263	12	.014	10	0	4	0	5	-.012	6
211		11	max	896.036	2	.327	2	7.007	4	0	10	.003	4	-.003	15
212			min	-1022.365	3	-.138	3	.014	10	0	4	0	10	-.012	6
213		12	max	895.866	2	-.141	15	7.612	4	0	10	.007	4	-.003	15
214			min	-1022.493	3	-.654	6	.014	10	0	4	0	10	-.012	6
215		13	max	895.695	2	-.345	15	8.217	4	0	10	.01	4	-.003	15
216			min	-1022.621	3	-1.523	6	.014	10	0	4	0	10	-.012	6
217		14	max	895.525	2	-.55	15	8.822	4	0	10	.014	4	-.002	15
218			min	-1022.748	3	-2.392	6	.014	10	0	4	0	10	-.011	6
219		15	max	895.355	2	-.754	15	9.427	4	0	10	.019	4	-.002	15
220			min	-1022.876	3	-3.26	6	.014	10	0	4	0	10	-.009	6
221		16	max	895.184	2	-.958	15	10.032	4	0	10	.023	4	-.002	15
222			min	-1023.004	3	-4.129	6	.014	10	0	4	0	10	-.008	6
223		17	max	895.014	2	-1.162	15	10.638	4	0	10	.028	4	-.001	15
224			min	-1023.132	3	-4.998	6	.014	10	0	4	0	10	-.006	6
225		18	max	894.844	2	-1.367	15	11.243	4	0	10	.033	4	0	15
226			min	-1023.26	3	-5.867	6	.014	10	0	4	0	10	-.003	6
227		19	max	894.673	2	-1.571	15	11.848	4	0	10	.039	4	0	1
228			min	-1023.387	3	-6.736	6	.014	10	0	4	0	10	0	1
229	M4	1	max	862.11	1	0	1	-.535	10	0	1	.031	4	0	1
230			min	-97.612	5	0	1	-223.778	4	0	1	0	10	0	1
231		2	max	862.28	1	0	1	-.535	10	0	1	.005	4	0	1
232			min	-97.532	5	0	1	-223.925	4	0	1	0	10	0	1
233		3	max	862.451	1	0	1	-.535	10	0	1	0	12	0	1
234			min	-97.453	5	0	1	-224.073	4	0	1	-.02	4	0	1
235		4	max	862.621	1	0	1	-.535	10	0	1	0	10	0	1
236			min	-97.373	5	0	1	-224.22	4	0	1	-.046	4	0	1
237		5	max	862.791	1	0	1	-.535	10	0	1	0	10	0	1
238			min	-97.294	5	0	1	-224.368	4	0	1	-.072	4	0	1
239		6	max	862.962	1	0	1	-.535	10	0	1	0	10	0	1
240			min	-97.214	5	0	1	-224.516	4	0	1	-.098	4	0	1
241		7	max	863.132	1	0	1	-.535	10	0	1	0	10	0	1
242			min	-97.135	5	0	1	-224.663	4	0	1	-.123	4	0	1
243		8	max	863.302	1	0	1	-.535	10	0	1	0	10	0	1
244			min	-97.055	5	0	1	-224.811	4	0	1	-.149	4	0	1
245		9	max	863.473	1	0	1	-.535	10	0	1	0	10	0	1
246			min	-96.976	5	0	1	-224.959	4	0	1	-.175	4	0	1



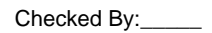
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	863.643	1	0	1	-.535	10	0	1	0	10	0	1
248		min	-96.896	5	0	1	-225.106	4	0	1	-.201	4	0	1
249	11	max	863.813	1	0	1	-.535	10	0	1	0	10	0	1
250		min	-96.817	5	0	1	-225.254	4	0	1	-.227	4	0	1
251	12	max	863.984	1	0	1	-.535	10	0	1	0	10	0	1
252		min	-96.737	5	0	1	-225.401	4	0	1	-.253	4	0	1
253	13	max	864.154	1	0	1	-.535	10	0	1	0	10	0	1
254		min	-96.658	5	0	1	-225.549	4	0	1	-.279	4	0	1
255	14	max	864.324	1	0	1	-.535	10	0	1	0	10	0	1
256		min	-96.578	5	0	1	-225.697	4	0	1	-.304	4	0	1
257	15	max	864.495	1	0	1	-.535	10	0	1	0	10	0	1
258		min	-96.499	5	0	1	-225.844	4	0	1	-.33	4	0	1
259	16	max	864.665	1	0	1	-.535	10	0	1	0	10	0	1
260		min	-96.419	5	0	1	-225.992	4	0	1	-.356	4	0	1
261	17	max	864.835	1	0	1	-.535	10	0	1	0	10	0	1
262		min	-96.34	5	0	1	-226.14	4	0	1	-.382	4	0	1
263	18	max	865.006	1	0	1	-.535	10	0	1	0	10	0	1
264		min	-96.26	5	0	1	-226.287	4	0	1	-.408	4	0	1
265	19	max	865.176	1	0	1	-.535	10	0	1	-.001	10	0	1
266		min	-96.181	5	0	1	-226.435	4	0	1	-.434	4	0	1
267	M6	1	max	3177.061	2	2.29	0	1	0	1	0	4	0	1
268		min	-4713.517	3	.228	12	-18.673	4	0	5	0	1	0	1
269	2	max	3177.59	2	2.235	2	0	1	0	1	0	1	0	12
270		min	-4713.12	3	.201	12	-19.134	4	0	5	-.007	4	0	2
271	3	max	3178.119	2	2.179	2	0	1	0	1	0	1	0	12
272		min	-4712.723	3	.166	3	-19.595	4	0	5	-.014	4	-.002	2
273	4	max	3178.649	2	2.124	2	0	1	0	1	0	1	0	12
274		min	-4712.326	3	.125	3	-20.056	4	0	5	-.021	4	-.002	2
275	5	max	3179.178	2	2.069	2	0	1	0	1	0	1	0	3
276		min	-4711.929	3	.083	3	-20.518	4	0	5	-.028	4	-.003	2
277	6	max	3179.707	2	2.013	2	0	1	0	1	0	1	0	3
278		min	-4711.532	3	.042	3	-20.979	4	0	5	-.036	4	-.004	2
279	7	max	3180.237	2	1.958	2	0	1	0	1	0	1	0	3
280		min	-4711.135	3	0	3	-21.44	4	0	5	-.043	4	-.005	2
281	8	max	3180.766	2	1.903	2	0	1	0	1	0	1	0	3
282		min	-4710.738	3	-.041	3	-21.901	4	0	5	-.051	4	-.005	2
283	9	max	3181.295	2	1.847	2	0	1	0	1	0	1	0	3
284		min	-4710.341	3	-.083	3	-22.362	4	0	5	-.059	4	-.006	2
285	10	max	3181.824	2	1.792	2	0	1	0	1	0	1	0	3
286		min	-4709.944	3	-.124	3	-22.824	4	0	5	-.067	4	-.007	2
287	11	max	3182.354	2	1.737	2	0	1	0	1	0	1	0	3
288		min	-4709.547	3	-.166	3	-23.285	4	0	5	-.075	4	-.007	2
289	12	max	3182.883	2	1.681	2	0	1	0	1	0	1	0	3
290		min	-4709.15	3	-.207	3	-23.746	4	0	5	-.084	4	-.008	2
291	13	max	3183.412	2	1.626	2	0	1	0	1	0	1	0	3
292		min	-4708.753	3	-.249	3	-24.207	4	0	5	-.092	4	-.008	2
293	14	max	3183.942	2	1.571	2	0	1	0	1	0	1	0	3
294		min	-4708.356	3	-.29	3	-24.669	4	0	5	-.101	4	-.009	2
295	15	max	3184.471	2	1.515	2	0	1	0	1	0	1	0	3
296		min	-4707.959	3	-.332	3	-25.13	4	0	5	-.11	4	-.01	2
297	16	max	3185	2	1.46	2	0	1	0	1	0	1	0	3
298		min	-4707.562	3	-.373	3	-25.591	4	0	5	-.119	4	-.01	2
299	17	max	3185.529	2	1.405	2	0	1	0	1	0	1	0	3
300		min	-4707.165	3	-.415	3	-26.052	4	0	5	-.128	4	-.011	2
301	18	max	3186.059	2	1.349	2	0	1	0	1	0	1	0	3
302		min	-4706.768	3	-.456	3	-26.513	4	0	5	-.138	4	-.011	2
303	19	max	3186.588	2	1.294	2	0	1	0	1	0	1	0	3



RISA-3D Version 13.0.0 \...\...\PVMMax 72 Cell 2V 30° 160mph 30psf 6.25ft 7-10Rade 25



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2390.839	2	0	1	0	1	0	1	0	1	0	1
362			min	-206.501	3	0	1	-216.021	4	0	1	-.194	4	0	1
363		11	max	2391.01	2	0	1	0	1	0	1	0	1	0	1
364			min	-206.373	3	0	1	-216.169	4	0	1	-.219	4	0	1
365		12	max	2391.18	2	0	1	0	1	0	1	0	1	0	1
366			min	-206.245	3	0	1	-216.316	4	0	1	-.244	4	0	1
367		13	max	2391.35	2	0	1	0	1	0	1	0	1	0	1
368			min	-206.118	3	0	1	-216.464	4	0	1	-.268	4	0	1
369		14	max	2391.521	2	0	1	0	1	0	1	0	1	0	1
370			min	-205.99	3	0	1	-216.611	4	0	1	-.293	4	0	1
371		15	max	2391.691	2	0	1	0	1	0	1	0	1	0	1
372			min	-205.862	3	0	1	-216.759	4	0	1	-.318	4	0	1
373		16	max	2391.861	2	0	1	0	1	0	1	0	1	0	1
374			min	-205.734	3	0	1	-216.907	4	0	1	-.343	4	0	1
375		17	max	2392.032	2	0	1	0	1	0	1	0	1	0	1
376			min	-205.607	3	0	1	-217.054	4	0	1	-.368	4	0	1
377		18	max	2392.202	2	0	1	0	1	0	1	0	1	0	1
378			min	-205.479	3	0	1	-217.202	4	0	1	-.393	4	0	1
379		19	max	2392.372	2	0	1	0	1	0	1	0	1	0	1
380			min	-205.351	3	0	1	-217.35	4	0	1	-.418	4	0	1
381	M10	1	max	1103.186	2	1.989	6	-.013	10	0	1	0	4	0	1
382			min	-1581.815	3	.451	15	-18.602	4	0	5	0	3	0	1
383		2	max	1103.715	2	1.918	6	-.013	10	0	1	0	10	0	15
384			min	-1581.418	3	.434	15	-19.063	4	0	5	-.007	4	0	6
385		3	max	1104.244	2	1.847	6	-.013	10	0	1	0	10	0	15
386			min	-1581.021	3	.418	15	-19.524	4	0	5	-.014	4	-.001	6
387		4	max	1104.774	2	1.776	6	-.013	10	0	1	0	10	0	15
388			min	-1580.624	3	.401	15	-19.986	4	0	5	-.021	4	-.002	6
389		5	max	1105.303	2	1.705	6	-.013	10	0	1	0	10	0	15
390			min	-1580.227	3	.384	15	-20.447	4	0	5	-.028	4	-.003	6
391		6	max	1105.832	2	1.634	6	-.013	10	0	1	0	10	0	15
392			min	-1579.83	3	.368	15	-20.908	4	0	5	-.035	4	-.003	6
393		7	max	1106.362	2	1.563	6	-.013	10	0	1	0	10	0	15
394			min	-1579.433	3	.351	15	-21.369	4	0	5	-.043	4	-.004	6
395		8	max	1106.891	2	1.492	6	-.013	10	0	1	0	10	0	15
396			min	-1579.036	3	.334	15	-21.831	4	0	5	-.051	4	-.004	6
397		9	max	1107.42	2	1.421	6	-.013	10	0	1	0	10	-.001	15
398			min	-1578.639	3	.318	15	-22.292	4	0	5	-.059	4	-.005	6
399		10	max	1107.949	2	1.35	6	-.013	10	0	1	0	10	-.001	15
400			min	-1578.242	3	.301	15	-22.753	4	0	5	-.067	4	-.005	6
401		11	max	1108.479	2	1.279	6	-.013	10	0	1	0	10	-.001	15
402			min	-1577.845	3	.284	15	-23.214	4	0	5	-.075	4	-.006	6
403		12	max	1109.008	2	1.208	6	-.013	10	0	1	0	10	-.001	15
404			min	-1577.448	3	.268	15	-23.675	4	0	5	-.083	4	-.006	6
405		13	max	1109.537	2	1.152	2	-.013	10	0	1	0	10	-.002	15
406			min	-1577.052	3	.251	15	-24.137	4	0	5	-.092	4	-.007	6
407		14	max	1110.067	2	1.096	2	-.013	10	0	1	0	10	-.002	15
408			min	-1576.655	3	.234	15	-24.598	4	0	5	-.101	4	-.007	6
409		15	max	1110.596	2	1.041	2	-.013	10	0	1	0	10	-.002	15
410			min	-1576.258	3	.207	12	-25.059	4	0	5	-.11	4	-.007	6
411		16	max	1111.125	2	.986	2	-.013	10	0	1	0	10	-.002	15
412			min	-1575.861	3	.179	12	-25.52	4	0	5	-.119	4	-.008	6
413		17	max	1111.654	2	.93	2	-.013	10	0	1	0	10	-.002	15
414			min	-1575.464	3	.152	12	-25.981	4	0	5	-.128	4	-.008	6
415		18	max	1112.184	2	.875	2	-.013	10	0	1	0	10	-.002	15
416			min	-1575.067	3	.124	12	-26.443	4	0	5	-.137	4	-.008	6
417		19	max	1112.713	2	.82	2	-.013	10	0	1	0	10	-.002	15

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1574.67	3	.096	12	-26.904	4	0	5	-.147	4	-.009	6
419	M11	1	max	897.74	2	8.849	6	.843	4	0	1	0	10	.009	6
420			min	-1021.088	3	2.068	15	-.178	1	0	4	-.016	4	.002	15
421		2	max	897.569	2	7.98	6	1.449	4	0	1	0	10	.005	2
422			min	-1021.215	3	1.863	15	-.178	1	0	4	-.015	4	0	12
423		3	max	897.399	2	7.111	6	2.054	4	0	1	0	10	.002	2
424			min	-1021.343	3	1.659	15	-.178	1	0	4	-.014	4	-.001	3
425		4	max	897.229	2	6.242	6	2.659	4	0	1	0	10	0	2
426			min	-1021.471	3	1.455	15	-.178	1	0	4	-.013	4	-.003	3
427		5	max	897.058	2	5.373	6	3.264	4	0	1	0	10	-.001	15
428			min	-1021.599	3	1.251	15	-.178	1	0	4	-.012	4	-.005	4
429		6	max	896.888	2	4.505	6	3.869	4	0	1	0	10	-.002	15
430			min	-1021.726	3	1.046	15	-.178	1	0	4	-.01	4	-.007	4
431		7	max	896.717	2	3.636	6	4.474	4	0	1	0	10	-.002	15
432			min	-1021.854	3	.842	15	-.178	1	0	4	-.008	4	-.009	4
433		8	max	896.547	2	2.767	6	5.079	4	0	1	0	10	-.003	15
434			min	-1021.982	3	.638	15	-.178	1	0	4	-.006	4	-.01	4
435		9	max	896.377	2	1.898	6	5.684	4	0	1	0	10	-.003	15
436			min	-1022.11	3	.434	15	-.178	1	0	4	-.003	4	-.012	4
437		10	max	896.206	2	1.029	6	6.289	4	0	1	0	10	-.003	15
438			min	-1022.237	3	.229	15	-.178	1	0	4	0	1	-.012	4
439		11	max	896.036	2	.327	2	6.894	4	0	1	.003	5	-.003	15
440			min	-1022.365	3	-.138	3	-.178	1	0	4	0	1	-.012	4
441		12	max	895.866	2	-.179	15	7.499	4	0	1	.006	5	-.003	15
442			min	-1022.493	3	-.71	4	-.178	1	0	4	-.001	1	-.012	4
443		13	max	895.695	2	-.383	15	8.104	4	0	1	.01	5	-.003	15
444			min	-1022.621	3	-1.579	4	-.178	1	0	4	-.001	1	-.012	4
445		14	max	895.525	2	-.588	15	8.709	4	0	1	.014	5	-.003	15
446			min	-1022.748	3	-2.448	4	-.178	1	0	4	-.001	1	-.011	4
447		15	max	895.355	2	-.792	15	9.314	4	0	1	.018	5	-.002	15
448			min	-1022.876	3	-3.317	4	-.178	1	0	4	-.001	1	-.01	4
449		16	max	895.184	2	-.996	15	9.92	4	0	1	.022	5	-.002	15
450			min	-1023.004	3	-4.185	4	-.178	1	0	4	-.001	1	-.008	4
451		17	max	895.014	2	-1.2	15	10.525	4	0	1	.027	5	-.001	15
452			min	-1023.132	3	-5.054	4	-.178	1	0	4	-.001	1	-.006	4
453		18	max	894.844	2	-1.405	15	11.13	4	0	1	.032	5	0	15
454			min	-1023.26	3	-5.923	4	-.178	1	0	4	-.002	1	-.003	4
455		19	max	894.673	2	-1.609	15	11.735	4	0	1	.038	5	0	1
456			min	-1023.387	3	-6.792	4	-.178	1	0	4	-.002	1	0	1
457	M12	1	max	862.11	1	0	1	6.745	1	0	1	.03	5	0	1
458			min	-33.779	3	0	1	-219.481	4	0	1	-.001	1	0	1
459		2	max	862.28	1	0	1	6.745	1	0	1	.005	5	0	1
460			min	-33.651	3	0	1	-219.629	4	0	1	0	1	0	1
461		3	max	862.451	1	0	1	6.745	1	0	1	0	1	0	1
462			min	-33.523	3	0	1	-219.776	4	0	1	-.02	4	0	1
463		4	max	862.621	1	0	1	6.745	1	0	1	.001	1	0	1
464			min	-33.396	3	0	1	-219.924	4	0	1	-.046	4	0	1
465		5	max	862.791	1	0	1	6.745	1	0	1	.002	1	0	1
466			min	-33.268	3	0	1	-220.072	4	0	1	-.071	4	0	1
467		6	max	862.962	1	0	1	6.745	1	0	1	.003	1	0	1
468			min	-33.14	3	0	1	-220.219	4	0	1	-.096	4	0	1
469		7	max	863.132	1	0	1	6.745	1	0	1	.003	1	0	1
470			min	-33.012	3	0	1	-220.367	4	0	1	-.121	4	0	1
471		8	max	863.302	1	0	1	6.745	1	0	1	.004	1	0	1
472			min	-32.884	3	0	1	-220.515	4	0	1	-.147	4	0	1
473		9	max	863.473	1	0	1	6.745	1	0	1	.005	1	0	1
474			min	-32.757	3	0	1	-220.662	4	0	1	-.172	4	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	863.643	1	0	1	6.745	1	0	1	.006	1	0	1
476			min	-32.629	3	0	1	-220.81	4	0	1	-.197	4	0	1
477		11	max	863.813	1	0	1	6.745	1	0	1	.006	1	0	1
478			min	-32.501	3	0	1	-220.958	4	0	1	-.223	4	0	1
479		12	max	863.984	1	0	1	6.745	1	0	1	.007	1	0	1
480			min	-32.373	3	0	1	-221.105	4	0	1	-.248	4	0	1
481		13	max	864.154	1	0	1	6.745	1	0	1	.008	1	0	1
482			min	-32.246	3	0	1	-221.253	4	0	1	-.273	4	0	1
483		14	max	864.324	1	0	1	6.745	1	0	1	.009	1	0	1
484			min	-32.118	3	0	1	-221.4	4	0	1	-.299	4	0	1
485		15	max	864.495	1	0	1	6.745	1	0	1	.01	1	0	1
486			min	-31.99	3	0	1	-221.548	4	0	1	-.324	4	0	1
487		16	max	864.665	1	0	1	6.745	1	0	1	.01	1	0	1
488			min	-31.862	3	0	1	-221.696	4	0	1	-.35	4	0	1
489		17	max	864.835	1	0	1	6.745	1	0	1	.011	1	0	1
490			min	-31.735	3	0	1	-221.843	4	0	1	-.375	4	0	1
491		18	max	865.006	1	0	1	6.745	1	0	1	.012	1	0	1
492			min	-31.607	3	0	1	-221.991	4	0	1	-.401	4	0	1
493		19	max	865.176	1	0	1	6.745	1	0	1	.013	1	0	1
494			min	-31.479	3	0	1	-222.139	4	0	1	-.426	4	0	1
495	M1	1	max	127.894	1	805.375	3	36.368	5	0	2	.103	1	0	15
496			min	-21.881	5	-417.806	2	-36.975	1	0	3	-.096	5	-.011	3
497		2	max	128.737	1	804.28	3	37.828	5	0	2	.08	1	.25	2
498			min	-21.487	5	-419.265	2	-36.975	1	0	3	-.073	5	-.51	3
499		3	max	660.743	3	580.087	2	21.22	5	0	3	.057	1	.499	2
500			min	-395.961	2	-649.32	3	-36.843	1	0	2	-.049	5	-.993	3
501		4	max	661.375	3	578.628	2	22.681	5	0	3	.034	1	.14	2
502			min	-395.118	2	-650.414	3	-36.843	1	0	2	-.035	5	-.59	3
503		5	max	662.006	3	577.169	2	24.141	5	0	3	.011	1	-.005	15
504			min	-394.276	2	-651.509	3	-36.843	1	0	2	-.021	5	-.219	2
505		6	max	662.638	3	575.71	2	25.601	5	0	3	0	10	.219	3
506			min	-393.434	2	-652.603	3	-36.843	1	0	2	-.012	1	-.577	2
507		7	max	663.27	3	574.251	2	27.061	5	0	3	.011	5	.624	3
508			min	-392.591	2	-653.697	3	-36.843	1	0	2	-.034	1	-.933	2
509		8	max	663.902	3	572.792	2	28.521	5	0	3	.028	5	1.03	3
510			min	-391.749	2	-654.792	3	-36.843	1	0	2	-.057	1	-1.289	2
511		9	max	680.165	3	53.174	2	49.488	5	0	9	.039	1	1.195	3
512			min	-338.38	2	.436	15	-64.123	1	0	3	-.106	5	-1.468	2
513		10	max	680.796	3	51.715	2	50.948	5	0	9	0	10	1.174	3
514			min	-337.538	2	-.01	5	-64.123	1	0	3	-.075	4	-1.501	2
515		11	max	681.428	3	50.256	2	52.409	5	0	9	-.003	10	1.152	3
516			min	-336.695	2	-1.858	4	-64.123	1	0	3	-.052	4	-1.532	2
517		12	max	697.163	3	448.536	3	127.752	5	0	2	.057	1	1.015	3
518			min	-283.078	2	-688.095	2	-35.9	1	0	3	-.215	5	-1.362	2
519		13	max	697.795	3	447.441	3	129.212	5	0	2	.034	1	.737	3
520			min	-282.236	2	-689.554	2	-35.9	1	0	3	-.135	5	-.935	2
521		14	max	698.427	3	446.347	3	130.672	5	0	2	.012	1	.46	3
522			min	-281.393	2	-691.013	2	-35.9	1	0	3	-.055	5	-.507	2
523		15	max	699.059	3	445.253	3	132.132	5	0	2	.027	5	.183	3
524			min	-280.551	2	-692.472	2	-35.9	1	0	3	-.01	1	-.086	1
525		16	max	699.691	3	444.158	3	133.592	5	0	2	.109	5	.353	2
526			min	-279.709	2	-693.931	2	-35.9	1	0	3	-.033	1	-.093	3
527		17	max	700.322	3	443.064	3	135.053	5	0	2	.193	5	.784	2
528			min	-278.866	2	-695.39	2	-35.9	1	0	3	-.055	1	-.368	3
529		18	max	37.742	5	644.783	2	-3.28	10	0	5	.193	5	.398	2
530			min	-129.492	1	-311.881	3	-81.992	4	0	2	-.079	1	-.183	3
531		19	max	38.135	5	643.324	2	-3.28	10	0	5	.149	5	.011	3





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	680.796	3	51.715	2	76.782	4	0	3	0	1	1.174	3
590		min	-337.538	2	.014	15	4.58	10	0	9	-.075	4	-1.501	2
591	11	max	681.428	3	50.256	2	78.242	4	0	3	.04	1	1.152	3
592		min	-336.695	2	-1.73	6	4.58	10	0	9	-.036	5	-1.532	2
593	12	max	697.163	3	448.536	3	143.162	4	0	3	-.004	10	1.015	3
594		min	-283.078	2	-688.095	2	2.765	10	0	2	-.239	4	-1.362	2
595	13	max	697.795	3	447.441	3	144.622	4	0	3	-.003	10	.737	3
596		min	-282.236	2	-689.554	2	2.765	10	0	2	-.15	4	-.935	2
597	14	max	698.427	3	446.347	3	146.083	4	0	3	0	10	.46	3
598		min	-281.393	2	-691.013	2	2.765	10	0	2	-.06	4	-.507	2
599	15	max	699.059	3	445.253	3	147.543	4	0	3	.031	4	.183	3
600		min	-280.551	2	-692.472	2	2.765	10	0	2	0	10	-.086	1
601	16	max	699.691	3	444.158	3	149.003	4	0	3	.123	4	.353	2
602		min	-279.709	2	-693.931	2	2.765	10	0	2	.003	10	-.093	3
603	17	max	700.322	3	443.064	3	150.463	4	0	3	.216	4	.784	2
604		min	-278.866	2	-695.39	2	2.765	10	0	2	.004	10	-.368	3
605	18	max	-9.005	12	644.783	2	42.33	1	0	2	.226	4	.398	2
606		min	-129.492	1	-311.881	3	-65.142	5	0	3	.006	10	-.183	3
607	19	max	-8.583	12	643.324	2	42.33	1	0	2	.192	4	.011	3
608		min	-128.649	1	-312.975	3	-63.682	5	0	3	.008	10	-.003	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.235	2	.013	3	1.624e-2	2	NC	1	NC	1
2				min	-.548	4	-.082	3	-.008	2	-5.663e-3	3	NC	1	NC
3		2	max	0	1	.198	2	.015	3	1.686e-2	2	NC	4	NC	1
4			min	-.548	4	.005	15	-.007	5	-4.991e-3	3	1407.677	3	NC	1
5		3	max	0	1	.171	2	.018	3	1.748e-2	2	NC	4	NC	2
6			min	-.548	4	.004	15	-.009	5	-4.32e-3	3	767.878	3	7775.076	1
7		4	max	0	1	.172	3	.026	1	1.809e-2	2	NC	4	NC	2
8			min	-.548	4	.004	15	-.008	5	-3.649e-3	3	589.878	3	5427.248	1
9		5	max	0	1	.196	3	.029	1	1.871e-2	2	NC	4	NC	2
10			min	-.548	4	.004	15	-.006	10	-2.977e-3	3	540.423	3	4848.071	1
11		6	max	0	1	.187	2	.029	3	1.932e-2	2	NC	4	NC	2
12			min	-.548	4	.004	15	-.007	10	-2.306e-3	3	564.876	3	5334.57	1
13		7	max	0	1	.222	2	.032	3	1.994e-2	2	NC	2	NC	2
14			min	-.548	4	.004	15	-.01	2	-1.634e-3	3	667.077	3	7600.032	1
15		8	max	0	1	.264	2	.034	3	2.055e-2	2	NC	4	NC	1
16			min	-.548	4	.005	15	-.017	2	-9.63e-4	3	890.97	3	6997.863	3
17		9	max	0	1	.299	2	.035	3	2.117e-2	2	NC	4	NC	1
18			min	-.548	4	.006	15	-.023	2	-2.916e-4	3	1303.209	3	6647.928	3
19		10	max	0	1	.315	2	.036	3	2.179e-2	2	NC	4	NC	1
20			min	-.548	4	.006	15	-.025	2	3.798e-4	3	1659.353	3	6547.475	3
21		11	max	0	10	.299	2	.035	3	2.117e-2	2	NC	4	NC	1
22			min	-.548	4	.006	15	-.023	2	-2.916e-4	3	1303.209	3	6647.928	3
23		12	max	0	10	.264	2	.034	3	2.055e-2	2	NC	4	NC	1
24			min	-.548	4	.005	15	-.017	2	-9.63e-4	3	890.97	3	6997.863	3
25		13	max	0	10	.222	2	.032	3	1.994e-2	2	NC	2	NC	2
26			min	-.548	4	.004	15	-.01	2	-1.634e-3	3	667.077	3	7600.032	1
27		14	max	0	10	.187	2	.029	3	1.932e-2	2	NC	4	NC	2
28			min	-.548	4	.003	15	-.007	10	-2.306e-3	3	564.876	3	5334.57	1
29		15	max	0	10	.196	3	.029	1	1.871e-2	2	NC	4	NC	2
30			min	-.548	4	.003	15	-.006	10	-2.977e-3	3	540.423	3	4848.071	1
31		16	max	0	10	.172	3	.026	1	1.809e-2	2	NC	4	NC	2
32			min	-.548	4	.003	15	-.005	10	-3.649e-3	3	589.878	3	5427.248	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	10	.171	2	.019	14	1.748e-2	2	NC	4	NC	2
34		min	-.548	4	.003	15	-.004	10	-4.32e-3	3	767.878	3	7722.324	4
35	18	max	0	10	.198	2	.015	3	1.686e-2	2	NC	4	NC	1
36		min	-.548	4	.003	15	-.006	2	-4.991e-3	3	1407.677	3	NC	1
37	19	max	0	10	.235	2	.013	3	1.624e-2	2	NC	1	NC	1
38		min	-.548	4	-.082	3	-.008	2	-5.663e-3	3	NC	1	NC	1
39	M14	1	max	0	.511	3	.011	3	8.711e-3	2	NC	1	NC	1
40		min	-.415	4	-.687	2	-.007	2	-7.483e-3	3	NC	1	NC	1
41	2	max	0	1	.664	3	.012	3	9.702e-3	2	NC	5	NC	1
42		min	-.415	4	-.838	2	-.011	5	-8.444e-3	3	976.217	3	NC	1
43	3	max	0	1	.803	3	.015	3	1.069e-2	2	NC	5	NC	1
44		min	-.415	4	-.976	2	-.015	5	-9.406e-3	3	513.842	3	8931.4	5
45	4	max	0	1	.915	3	.02	1	1.168e-2	2	NC	5	NC	2
46		min	-.415	4	-1.094	2	-.011	5	-1.037e-2	3	368.469	2	6836.74	1
47	5	max	0	1	.994	3	.024	1	1.267e-2	2	NC	5	NC	2
48		min	-.415	4	-1.186	2	-.005	10	-1.133e-2	3	300.838	2	5806.964	1
49	6	max	0	1	1.039	3	.025	3	1.367e-2	2	NC	5	NC	2
50		min	-.415	4	-1.249	2	-.007	10	-1.229e-2	3	267.026	2	6181.164	1
51	7	max	0	1	1.053	3	.028	3	1.466e-2	2	NC	15	NC	2
52		min	-.415	4	-1.285	2	-.009	2	-1.325e-2	3	250.916	2	8072.042	14
53	8	max	0	1	1.044	3	.03	3	1.565e-2	2	NC	15	NC	1
54		min	-.415	4	-1.299	2	-.015	2	-1.421e-2	3	245.296	2	7545.728	4
55	9	max	0	1	1.026	3	.031	3	1.664e-2	2	NC	15	NC	1
56		min	-.415	4	-1.298	2	-.021	2	-1.517e-2	3	245.528	2	7552.448	3
57	10	max	0	1	1.015	3	.031	3	1.763e-2	2	NC	15	NC	1
58		min	-.415	4	-1.295	2	-.023	2	-1.613e-2	3	246.888	2	7415.953	3
59	11	max	0	10	1.026	3	.031	3	1.664e-2	2	NC	15	NC	1
60		min	-.415	4	-1.298	2	-.021	2	-1.517e-2	3	245.528	2	7552.448	3
61	12	max	0	10	1.044	3	.03	3	1.565e-2	2	NC	15	NC	1
62		min	-.415	4	-1.299	2	-.015	2	-1.421e-2	3	245.296	2	8020.151	3
63	13	max	0	10	1.053	3	.028	3	1.466e-2	2	NC	15	NC	2
64		min	-.415	4	-1.285	2	-.01	5	-1.325e-2	3	250.916	2	8587.519	1
65	14	max	0	10	1.039	3	.025	3	1.367e-2	2	NC	5	NC	2
66		min	-.415	4	-1.249	2	-.007	10	-1.229e-2	3	267.026	2	6181.164	1
67	15	max	0	10	.994	3	.024	1	1.267e-2	2	NC	5	NC	2
68		min	-.415	4	-1.186	2	-.005	10	-1.133e-2	3	300.838	2	5806.964	1
69	16	max	0	10	.915	3	.021	14	1.168e-2	2	NC	5	NC	2
70		min	-.415	4	-1.094	2	-.004	10	-1.037e-2	3	368.469	2	6836.74	1
71	17	max	0	10	.803	3	.022	4	1.069e-2	2	NC	5	NC	1
72		min	-.415	4	-.976	2	-.004	10	-9.406e-3	3	513.842	3	6616.698	4
73	18	max	0	10	.664	3	.015	4	9.702e-3	2	NC	5	NC	1
74		min	-.415	4	-.838	2	-.006	2	-8.444e-3	3	976.217	3	9466.264	4
75	19	max	0	10	.511	3	.011	3	8.711e-3	2	NC	1	NC	1
76		min	-.415	4	-.687	2	-.007	2	-7.483e-3	3	NC	1	NC	1
77	M15	1	max	0	.521	3	.01	3	6.445e-3	3	NC	1	NC	1
78		min	-.342	4	-.685	2	-.007	2	-9.089e-3	2	NC	1	NC	1
79	2	max	0	10	.645	3	.012	3	7.259e-3	3	NC	5	NC	1
80		min	-.342	4	-.861	2	-.017	5	-1.013e-2	2	853.079	2	7821.396	5
81	3	max	0	10	.758	3	.014	3	8.072e-3	3	NC	5	NC	1
82		min	-.342	4	-1.02	2	-.022	5	-1.117e-2	2	447.851	2	6246.21	5
83	4	max	0	10	.856	3	.021	1	8.885e-3	3	NC	5	NC	2
84		min	-.342	4	-1.151	2	-.017	5	-1.222e-2	2	322.419	2	6776.355	1
85	5	max	0	10	.933	3	.025	1	9.698e-3	3	NC	5	NC	2
86		min	-.342	4	-1.245	2	-.006	5	-1.326e-2	2	267.97	2	5748.576	1
87	6	max	0	10	.987	3	.023	3	1.051e-2	3	NC	5	NC	2
88		min	-.342	4	-1.302	2	-.006	10	-1.43e-2	2	243.249	2	6099.596	1
89	7	max	0	10	1.02	3	.026	3	1.132e-2	3	NC	15	NC	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.085	1	.025	4	1.397e-2	3	NC	4	NC	2
148			min	-.11	4	-.15	3	-.003	10	-1.388e-2	2	1199.044	2	5808.415	4
149		18	max	0	1	.139	2	.016	4	1.331e-2	3	NC	4	NC	1
150			min	-.11	4	-.165	3	-.003	2	-1.379e-2	2	2155.102	2	8644.992	4
151		19	max	0	1	.209	2	.009	3	1.266e-2	3	NC	1	NC	1
152			min	-.11	4	-.187	3	-.006	2	-1.369e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.013	2	.005	1	1.809e-3	5	NC	1	NC	1
154			min	-.012	3	-.019	3	-.517	4	-1.03e-4	1	6097.475	2	149.951	4
155		2	max	.008	2	.011	2	.004	1	1.823e-3	5	NC	1	NC	1
156			min	-.011	3	-.018	3	-.475	4	-9.806e-5	1	7039.053	2	163.131	4
157		3	max	.007	2	.009	2	.004	1	1.836e-3	5	NC	1	NC	1
158			min	-.01	3	-.018	3	-.433	4	-9.308e-5	1	8305.773	2	178.76	4
159		4	max	.007	2	.008	2	.004	1	1.849e-3	5	NC	1	NC	1
160			min	-.01	3	-.017	3	-.392	4	-8.811e-5	1	NC	1	197.477	4
161		5	max	.006	2	.006	2	.003	1	1.863e-3	5	NC	1	NC	1
162			min	-.009	3	-.016	3	-.352	4	-8.313e-5	1	NC	1	220.145	4
163		6	max	.006	2	.005	2	.003	1	1.876e-3	5	NC	1	NC	1
164			min	-.008	3	-.016	3	-.312	4	-7.816e-5	1	NC	1	247.966	4
165		7	max	.006	2	.003	2	.003	1	1.89e-3	5	NC	1	NC	1
166			min	-.008	3	-.015	3	-.274	4	-7.318e-5	1	NC	1	282.637	4
167		8	max	.005	2	.002	2	.002	1	1.903e-3	5	NC	1	NC	1
168			min	-.007	3	-.014	3	-.237	4	-6.82e-5	1	NC	1	326.633	4
169		9	max	.005	2	0	2	.002	1	1.917e-3	5	NC	1	NC	1
170			min	-.007	3	-.013	3	-.202	4	-6.323e-5	1	NC	1	383.677	4
171		10	max	.004	2	0	2	.002	1	1.931e-3	4	NC	1	NC	1
172			min	-.006	3	-.012	3	-.169	4	-5.825e-5	1	NC	1	459.579	4
173		11	max	.004	2	-.001	2	.001	1	1.945e-3	4	NC	1	NC	1
174			min	-.005	3	-.011	3	-.137	4	-5.328e-5	1	NC	1	563.827	4
175		12	max	.003	2	-.001	15	0	1	1.96e-3	4	NC	1	NC	1
176			min	-.005	3	-.01	3	-.109	4	-4.83e-5	1	NC	1	712.812	4
177		13	max	.003	2	-.001	15	0	1	1.975e-3	4	NC	1	NC	1
178			min	-.004	3	-.009	3	-.083	4	-4.333e-5	1	NC	1	936.894	4
179		14	max	.002	2	-.001	15	0	1	1.99e-3	4	NC	1	NC	1
180			min	-.003	3	-.008	3	-.06	4	-3.835e-5	1	NC	1	1297.694	4
181		15	max	.002	2	-.001	15	0	1	2.005e-3	4	NC	1	NC	1
182			min	-.003	3	-.006	3	-.04	4	-3.337e-5	1	NC	1	1936.839	4
183		16	max	.001	2	0	15	0	1	2.019e-3	4	NC	1	NC	1
184			min	-.002	3	-.005	3	-.024	4	-2.84e-5	1	NC	1	3244.264	4
185		17	max	0	2	0	15	0	1	2.034e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	3	-.012	4	-2.342e-5	1	NC	1	6656.953	4
187		18	max	0	2	0	15	0	1	2.049e-3	4	NC	1	NC	1
188			min	0	3	-.002	6	-.004	4	-1.845e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.064e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.347e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	2.77e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.465e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.011	4	5.841e-5	4	NC	1	NC	1
194			min	0	2	-.003	6	0	1	1.158e-6	10	NC	1	9206.523	4
195		3	max	.001	3	-.001	15	.021	4	5.633e-4	4	NC	1	NC	1
196			min	0	2	-.006	6	0	1	2.166e-6	10	NC	1	4787.804	4
197		4	max	.002	3	-.002	15	.031	4	1.068e-3	4	NC	1	NC	1
198			min	-.001	2	-.009	6	0	1	3.175e-6	10	NC	1	3319.786	4
199		5	max	.002	3	-.003	15	.039	4	1.573e-3	4	NC	1	NC	1
200			min	-.002	2	-.012	6	0	1	4.183e-6	10	8550.158	6	2588.355	4
201		6	max	.003	3	-.003	15	.047	4	2.078e-3	4	NC	2	NC	1
202			min	-.002	2	-.015	6	0	1	5.191e-6	10	6926.451	6	2150.382	4
203		7	max	.003	3	-.004	15	.055	4	2.583e-3	4	NC	5	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-0.003	2	-0.017	6	0	1	6.199e-6	10	5948.632	6	1857.869	4
205		8	max	.004	3	-0.004	15	.062	4	3.088e-3	4	NC	5	NC	1
206			min	-0.003	2	-0.019	6	0	1	7.207e-6	10	5345.531	6	1647.121	4
207		9	max	.004	3	-0.004	15	.068	4	3.593e-3	4	NC	5	NC	1
208			min	-0.004	2	-.02	6	0	3	8.215e-6	10	4989.515	6	1486.06	4
209		10	max	.005	3	-0.005	15	.075	4	4.098e-3	4	NC	5	NC	1
210			min	-.004	2	-.021	6	0	3	9.224e-6	10	4818.693	6	1356.684	4
211		11	max	.006	3	-0.005	15	.081	4	4.603e-3	4	NC	5	NC	1
212			min	-0.005	2	-0.021	6	0	12	1.023e-5	10	4807.942	6	1248.081	4
213		12	max	.006	3	-0.004	15	.088	4	5.108e-3	4	NC	5	NC	1
214			min	-0.005	2	-.02	6	0	12	1.124e-5	10	4959.276	6	1153.281	4
215		13	max	.007	3	-0.004	15	.095	4	5.613e-3	4	NC	5	NC	1
216			min	-0.006	2	-0.019	6	0	10	1.225e-5	10	5303.858	6	1067.704	4
217		14	max	.007	3	-0.004	15	.103	4	6.117e-3	4	NC	5	NC	1
218			min	-0.006	2	-0.017	6	0	10	1.326e-5	10	5918.101	6	988.34	4
219		15	max	.008	3	-0.003	15	.111	4	6.622e-3	4	NC	3	NC	1
220			min	-.007	2	-0.014	6	0	10	1.426e-5	10	6971.159	6	913.288	4
221		16	max	.008	3	-0.002	15	.121	4	7.127e-3	4	NC	1	NC	1
222			min	-0.007	2	-0.011	6	0	10	1.527e-5	10	8872.553	6	841.452	4
223		17	max	.009	3	-0.001	15	.132	4	7.632e-3	4	NC	1	NC	1
224			min	-0.008	2	-0.008	6	0	10	1.628e-5	10	NC	1	772.334	4
225		18	max	.009	3	0	15	.144	4	8.137e-3	4	NC	1	NC	1
226			min	-0.008	2	-0.005	3	0	10	1.729e-5	10	NC	1	705.858	4
227		19	max	.01	3	0	5	.158	4	8.642e-3	4	NC	1	NC	1
228			min	-0.009	2	-0.002	3	0	10	1.83e-5	10	NC	1	642.219	4
229	M4	1	max	.002	1	.009	2	0	10	5.219e-4	4	NC	1	NC	2
230			min	0	5	-.01	3	-.158	4	6.641e-6	10	NC	1	156.697	4
231		2	max	.002	1	.008	2	0	10	5.219e-4	4	NC	1	NC	2
232			min	0	5	-.01	3	-.146	4	6.641e-6	10	NC	1	170.223	4
233		3	max	.002	1	.008	2	0	10	5.219e-4	4	NC	1	NC	2
234			min	0	5	-0.009	3	-.133	4	6.641e-6	10	NC	1	186.331	4
235		4	max	.002	1	.007	2	0	10	5.219e-4	4	NC	1	NC	2
236			min	0	5	-0.009	3	-.121	4	6.641e-6	10	NC	1	205.687	4
237		5	max	.002	1	.007	2	0	10	5.219e-4	4	NC	1	NC	2
238			min	0	5	-0.008	3	-.108	4	6.641e-6	10	NC	1	229.202	4
239		6	max	.001	1	.006	2	0	10	5.219e-4	4	NC	1	NC	2
240			min	0	5	-0.007	3	-0.096	4	6.641e-6	10	NC	1	258.135	4
241		7	max	.001	1	.006	2	0	10	5.219e-4	4	NC	1	NC	1
242			min	0	5	-0.007	3	-.084	4	6.641e-6	10	NC	1	294.276	4
243		8	max	.001	1	.005	2	0	10	5.219e-4	4	NC	1	NC	1
244			min	0	5	-0.006	3	-0.073	4	6.641e-6	10	NC	1	340.232	4
245		9	max	.001	1	.005	2	0	10	5.219e-4	4	NC	1	NC	1
246			min	0	5	-0.006	3	-.062	4	6.641e-6	10	NC	1	399.93	4
247		10	max	.001	1	.004	2	0	10	5.219e-4	4	NC	1	NC	1
248			min	0	5	-0.005	3	-.052	4	6.641e-6	10	NC	1	479.509	4
249		11	max	0	1	.004	2	0	10	5.219e-4	4	NC	1	NC	1
250			min	0	5	-0.005	3	-.042	4	6.641e-6	10	NC	1	589.005	4
251		12	max	0	1	.003	2	0	10	5.219e-4	4	NC	1	NC	1
252			min	0	5	-0.004	3	-.033	4	6.641e-6	10	NC	1	745.779	4
253		13	max	0	1	.003	2	0	10	5.219e-4	4	NC	1	NC	1
254			min	0	5	-0.003	3	-.025	4	6.641e-6	10	NC	1	982.048	4
255		14	max	0	1	.002	2	0	10	5.219e-4	4	NC	1	NC	1
256			min	0	5	-0.003	3	-.018	4	6.641e-6	10	NC	1	1363.331	4
257		15	max	0	1	.002	2	0	10	5.219e-4	4	NC	1	NC	1
258			min	0	5	-0.002	3	-.012	4	6.641e-6	10	NC	1	2040.617	4
259		16	max	0	1	.001	2	0	10	5.219e-4	4	NC	1	NC	1
260			min	0	5	-0.002	3	-.007	4	6.641e-6	10	NC	1	3431.116	4



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Dec 1, 2015

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	10	5.219e-4	4	NC	1	NC	1
262		min	0	5	-0.001	3	-0.004	4	6.641e-6	10	NC	1	7080.809	4
263	18	max	0	1	0	2	0	10	5.219e-4	4	NC	1	NC	1
264		min	0	5	0	3	-0.001	4	6.641e-6	10	NC	1	NC	1
265	19	max	0	1	0	1	0	1	5.219e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	6.641e-6	10	NC	1	NC	1
267	M6	1	max	.024	2	.038	2	0	1.876e-3	4	NC	3	NC	1
268		min	-.035	3	-.055	3	-.522	4	0	1	2018.888	2	148.517	4
269	2	max	.022	2	.035	2	0	1	1.889e-3	4	NC	3	NC	1
270		min	-.033	3	-.052	3	-.48	4	0	1	2216.834	2	161.572	4
271	3	max	.021	2	.032	2	0	1	1.902e-3	4	NC	3	NC	1
272		min	-.031	3	-.049	3	-.438	4	0	1	2455.741	2	177.052	4
273	4	max	.02	2	.028	2	0	1	1.914e-3	4	NC	3	NC	1
274		min	-.029	3	-.046	3	-.396	4	0	1	2747.227	2	195.59	4
275	5	max	.018	2	.025	2	0	1	1.927e-3	4	NC	3	NC	1
276		min	-.027	3	-.043	3	-.355	4	0	1	3107.477	2	218.043	4
277	6	max	.017	2	.022	2	0	1	1.94e-3	4	NC	3	NC	1
278		min	-.025	3	-.04	3	-.316	4	0	1	3559.591	2	245.597	4
279	7	max	.016	2	.019	2	0	1	1.953e-3	4	NC	1	NC	1
280		min	-.023	3	-.037	3	-.277	4	0	1	4137.464	2	279.936	4
281	8	max	.014	2	.016	2	0	1	1.965e-3	4	NC	1	NC	1
282		min	-.021	3	-.033	3	-.24	4	0	1	4892.489	2	323.511	4
283	9	max	.013	2	.013	2	0	1	1.978e-3	4	NC	1	NC	1
284		min	-.02	3	-.03	3	-.204	4	0	1	5905.703	2	380.006	4
285	10	max	.012	2	.011	2	0	1	1.991e-3	4	NC	1	NC	1
286		min	-.018	3	-.027	3	-.17	4	0	1	7311.122	2	455.173	4
287	11	max	.011	2	.008	2	0	1	2.004e-3	4	NC	1	NC	1
288		min	-.016	3	-.024	3	-.139	4	0	1	9343.876	2	558.407	4
289	12	max	.009	2	.006	2	0	1	2.017e-3	4	NC	1	NC	1
290		min	-.014	3	-.021	3	-.11	4	0	1	NC	1	705.932	4
291	13	max	.008	2	.004	2	0	1	2.029e-3	4	NC	1	NC	1
292		min	-.012	3	-.018	3	-.084	4	0	1	NC	1	927.796	4
293	14	max	.007	2	.003	2	0	1	2.042e-3	4	NC	1	NC	1
294		min	-.01	3	-.015	3	-.06	4	0	1	NC	1	1284.975	4
295	15	max	.005	2	.002	2	0	1	2.055e-3	4	NC	1	NC	1
296		min	-.008	3	-.012	3	-.04	4	0	1	NC	1	1917.581	4
297	16	max	.004	2	0	2	0	1	2.068e-3	4	NC	1	NC	1
298		min	-.006	3	-.009	3	-.024	4	0	1	NC	1	3211.23	4
299	17	max	.003	2	0	2	0	1	2.08e-3	4	NC	1	NC	1
300		min	-.004	3	-.006	3	-.012	4	0	1	NC	1	6586.156	4
301	18	max	.001	2	0	2	0	1	2.093e-3	4	NC	1	NC	1
302		min	-.002	3	-.003	3	-.004	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	2.106e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-4.563e-4	4	NC	1	NC	1
307	2	max	.002	3	0	15	.011	4	3.451e-5	4	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	9021.181	4
309	3	max	.003	3	-.001	15	.022	4	5.253e-4	4	NC	1	NC	1
310		min	-.003	2	-.008	3	0	1	0	1	NC	1	4691.266	4
311	4	max	.005	3	-.002	15	.031	4	1.016e-3	4	NC	1	NC	1
312		min	-.004	2	-.011	3	0	1	0	1	NC	1	3253.77	4
313	5	max	.006	3	-.003	15	.04	4	1.507e-3	4	NC	1	NC	1
314		min	-.006	2	-.014	3	0	1	0	1	7981.222	3	2538.52	4
315	6	max	.008	3	-.004	15	.048	4	1.998e-3	4	NC	1	NC	1
316		min	-.007	2	-.017	3	0	1	0	1	6738.4	3	2111.207	4
317	7	max	.009	3	-.004	15	.056	4	2.488e-3	4	NC	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.009	2	-.019	3	0	1	0	1	5914.787	4	1826.789	4
319	8	max	.011	3	-.005	15	.063	4	2.979e-3	4	NC	2	NC	1
320		min	-.01	2	-.021	3	0	1	0	1	5317.266	4	1622.844	4
321	9	max	.013	3	-.005	15	.069	4	3.47e-3	4	NC	5	NC	1
322		min	-.012	2	-.022	3	0	1	0	1	4964.804	4	1467.921	4
323	10	max	.014	3	-.005	15	.076	4	3.961e-3	4	NC	5	NC	1
324		min	-.013	2	-.023	3	0	1	0	1	4796.184	4	1344.338	4
325	11	max	.016	3	-.005	15	.082	4	4.451e-3	4	NC	5	NC	1
326		min	-.015	2	-.023	3	0	1	0	1	4786.624	4	1241.333	4
327	12	max	.017	3	-.005	15	.088	4	4.942e-3	4	NC	5	NC	1
328		min	-.016	2	-.022	3	0	1	0	1	4938.281	4	1151.969	4
329	13	max	.019	3	-.005	15	.095	4	5.433e-3	4	NC	5	NC	1
330		min	-.018	2	-.021	3	0	1	0	1	5282.299	4	1071.62	4
331	14	max	.02	3	-.004	15	.102	4	5.924e-3	4	NC	2	NC	1
332		min	-.019	2	-.02	3	0	1	0	1	5894.875	4	997.175	4
333	15	max	.022	3	-.004	15	.11	4	6.414e-3	4	NC	1	NC	1
334		min	-.021	2	-.018	3	0	1	0	1	6944.598	4	926.593	4
335	16	max	.024	3	-.003	15	.118	4	6.905e-3	4	NC	1	NC	1
336		min	-.022	2	-.015	3	0	1	0	1	8839.546	4	858.638	4
337	17	max	.025	3	-.002	15	.128	4	7.396e-3	4	NC	1	NC	1
338		min	-.024	2	-.013	3	0	1	0	1	NC	1	792.687	4
339	18	max	.027	3	-.001	15	.14	4	7.887e-3	4	NC	1	NC	1
340		min	-.025	2	-.01	3	0	1	0	1	NC	1	728.581	4
341	19	max	.028	3	0	10	.153	4	8.378e-3	4	NC	1	NC	1
342		min	-.027	2	-.007	3	0	1	0	1	NC	1	666.486	4
343	M8	1	max	.006	2	.026	2	0	3.538e-4	4	NC	1	NC	1
344		min	0	3	-.029	3	-.153	4	0	1	NC	1	162.618	4
345	2	max	.005	2	.025	2	0	1	3.538e-4	4	NC	1	NC	1
346		min	0	3	-.027	3	-.14	4	0	1	NC	1	176.677	4
347	3	max	.005	2	.023	2	0	1	3.538e-4	4	NC	1	NC	1
348		min	0	3	-.026	3	-.128	4	0	1	NC	1	193.417	4
349	4	max	.005	2	.022	2	0	1	3.538e-4	4	NC	1	NC	1
350		min	0	3	-.024	3	-.116	4	0	1	NC	1	213.533	4
351	5	max	.004	2	.02	2	0	1	3.538e-4	4	NC	1	NC	1
352		min	0	3	-.023	3	-.104	4	0	1	NC	1	237.969	4
353	6	max	.004	2	.019	2	0	1	3.538e-4	4	NC	1	NC	1
354		min	0	3	-.021	3	-.093	4	0	1	NC	1	268.035	4
355	7	max	.004	2	.017	2	0	1	3.538e-4	4	NC	1	NC	1
356		min	0	3	-.019	3	-.081	4	0	1	NC	1	305.59	4
357	8	max	.003	2	.016	2	0	1	3.538e-4	4	NC	1	NC	1
358		min	0	3	-.018	3	-.07	4	0	1	NC	1	353.344	4
359	9	max	.003	2	.014	2	0	1	3.538e-4	4	NC	1	NC	1
360		min	0	3	-.016	3	-.06	4	0	1	NC	1	415.377	4
361	10	max	.003	2	.013	2	0	1	3.538e-4	4	NC	1	NC	1
362		min	0	3	-.014	3	-.05	4	0	1	NC	1	498.069	4
363	11	max	.003	2	.012	2	0	1	3.538e-4	4	NC	1	NC	1
364		min	0	3	-.013	3	-.041	4	0	1	NC	1	611.85	4
365	12	max	.002	2	.01	2	0	1	3.538e-4	4	NC	1	NC	1
366		min	0	3	-.011	3	-.032	4	0	1	NC	1	774.762	4
367	13	max	.002	2	.009	2	0	1	3.538e-4	4	NC	1	NC	1
368		min	0	3	-.01	3	-.024	4	0	1	NC	1	1020.286	4
369	14	max	.002	2	.007	2	0	1	3.538e-4	4	NC	1	NC	1
370		min	0	3	-.008	3	-.018	4	0	1	NC	1	1416.515	4
371	15	max	.001	2	.006	2	0	1	3.538e-4	4	NC	1	NC	1
372		min	0	3	-.006	3	-.012	4	0	1	NC	1	2120.374	4
373	16	max	0	2	.004	2	0	1	3.538e-4	4	NC	1	NC	1
374		min	0	3	-.005	3	-.007	4	0	1	NC	1	3565.488	4



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.003	2	0	1	3.538e-4	4	NC	1	NC	1
376			min	0	3	-.003	3	-.003	4	0	1	NC	1	7358.754	4
377		18	max	0	2	.001	2	0	1	3.538e-4	4	NC	1	NC	1
378			min	0	3	-.002	3	-.001	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	3.538e-4	4	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	2	.013	2	0	10	1.864e-3	4	NC	1	NC	1
382			min	-.012	3	-.019	3	-.52	4	5.701e-6	10	6097.475	2	149.063	4
383		2	max	.008	2	.011	2	0	10	1.876e-3	4	NC	1	NC	1
384			min	-.011	3	-.018	3	-.478	4	5.426e-6	10	7039.053	2	162.167	4
385		3	max	.007	2	.009	2	0	10	1.887e-3	4	NC	1	NC	1
386			min	-.01	3	-.018	3	-.436	4	5.151e-6	10	8305.773	2	177.706	4
387		4	max	.007	2	.008	2	0	10	1.899e-3	4	NC	1	NC	1
388			min	-.01	3	-.017	3	-.395	4	4.876e-6	10	NC	1	196.314	4
389		5	max	.006	2	.006	2	0	10	1.91e-3	4	NC	1	NC	1
390			min	-.009	3	-.016	3	-.354	4	4.601e-6	10	NC	1	218.852	4
391		6	max	.006	2	.005	2	0	10	1.922e-3	4	NC	1	NC	1
392			min	-.008	3	-.016	3	-.314	4	4.325e-6	10	NC	1	246.513	4
393		7	max	.006	2	.003	2	0	10	1.933e-3	4	NC	1	NC	1
394			min	-.008	3	-.015	3	-.276	4	4.05e-6	10	NC	1	280.985	4
395		8	max	.005	2	.002	2	0	10	1.945e-3	4	NC	1	NC	1
396			min	-.007	3	-.014	3	-.239	4	3.775e-6	10	NC	1	324.73	4
397		9	max	.005	2	0	2	0	10	1.957e-3	4	NC	1	NC	1
398			min	-.007	3	-.013	3	-.203	4	3.5e-6	10	NC	1	381.45	4
399		10	max	.004	2	0	2	0	10	1.968e-3	4	NC	1	NC	1
400			min	-.006	3	-.012	3	-.17	4	3.225e-6	10	NC	1	456.92	4
401		11	max	.004	2	-.001	2	0	10	1.98e-3	4	NC	1	NC	1
402			min	-.005	3	-.011	3	-.138	4	2.95e-6	10	NC	1	560.578	4
403		12	max	.003	2	-.002	2	0	10	1.991e-3	4	NC	1	NC	1
404			min	-.005	3	-.01	3	-.109	4	2.674e-6	10	NC	1	708.723	4
405		13	max	.003	2	-.002	15	0	10	2.003e-3	4	NC	1	NC	1
406			min	-.004	3	-.009	3	-.083	4	2.399e-6	10	NC	1	931.546	4
407		14	max	.002	2	-.002	15	0	10	2.015e-3	4	NC	1	NC	1
408			min	-.003	3	-.008	3	-.06	4	2.124e-6	10	NC	1	1290.33	4
409		15	max	.002	2	-.002	15	0	10	2.026e-3	4	NC	1	NC	1
410			min	-.003	3	-.006	3	-.04	4	1.849e-6	10	NC	1	1925.929	4
411		16	max	.001	2	-.001	15	0	10	2.038e-3	4	NC	1	NC	1
412			min	-.002	3	-.005	4	-.024	4	1.574e-6	10	NC	1	3226.171	4
413		17	max	0	2	0	15	0	10	2.049e-3	4	NC	1	NC	1
414			min	-.001	3	-.004	4	-.012	4	1.299e-6	10	NC	1	6620.407	4
415		18	max	0	2	0	15	0	10	2.061e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.004	4	1.023e-6	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.072e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	7.483e-7	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-1.5e-7	10	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.483e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.011	4	5.067e-5	5	NC	1	NC	1
422			min	0	2	-.003	4	0	10	-1.511e-5	1	NC	1	9169.433	4
423		3	max	.001	3	-.002	15	.021	4	5.459e-4	4	NC	1	NC	1
424			min	0	2	-.006	4	0	10	-2.745e-5	1	NC	1	4769.557	4
425		4	max	.002	3	-.002	15	.031	4	1.043e-3	4	NC	1	NC	1
426			min	-.001	2	-.009	4	0	10	-3.979e-5	1	NC	1	3308.26	4
427		5	max	.002	3	-.003	15	.039	4	1.54e-3	4	NC	1	NC	1
428			min	-.002	2	-.013	4	0	10	-5.213e-5	1	8241.171	4	2580.6	4
429		6	max	.003	3	-.004	15	.047	4	2.037e-3	4	NC	2	NC	1
430			min	-.002	2	-.016	4	0	10	-6.447e-5	1	6697.707	4	2145.288	4
431		7	max	.003	3	-.004	15	.055	4	2.534e-3	4	NC	5	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.003	2	-.018	4	0	10	-7.682e-5	1	5767.297	4	1854.941	4
433		8	max	.004	3	-.005	15	.062	4	3.031e-3	4	NC	5	NC	1
434			min	-.003	2	-.02	4	0	10	-8.916e-5	1	5193.825	4	1646.125	4
435		9	max	.004	3	-.005	15	.068	4	3.528e-3	4	NC	5	NC	1
436			min	-.004	2	-.022	4	0	1	-1.015e-4	1	4856.689	4	1486.882	4
437		10	max	.005	3	-.006	15	.075	4	4.025e-3	4	NC	5	NC	1
438			min	-.004	2	-.022	4	0	1	-1.138e-4	1	4697.553	4	1359.262	4
439		11	max	.006	3	-.006	15	.081	4	4.522e-3	4	NC	5	NC	1
440			min	-.005	2	-.022	4	0	1	-1.262e-4	1	4693.095	4	1252.364	4
441		12	max	.006	3	-.005	15	.088	4	5.019e-3	4	NC	5	NC	1
442			min	-.005	2	-.022	4	0	1	-1.385e-4	1	4846.068	4	1159.201	4
443		13	max	.007	3	-.005	15	.095	4	5.516e-3	4	NC	5	NC	1
444			min	-.006	2	-.021	4	-.001	1	-1.509e-4	1	5187.517	4	1075.155	4
445		14	max	.007	3	-.005	15	.102	4	6.013e-3	4	NC	5	NC	1
446			min	-.006	2	-.019	4	-.001	1	-1.632e-4	1	5792.689	4	997.17	4
447		15	max	.008	3	-.004	15	.11	4	6.511e-3	4	NC	3	NC	1
448			min	-.007	2	-.016	4	-.002	1	-1.755e-4	1	6827.668	4	923.29	4
449		16	max	.008	3	-.003	15	.119	4	7.008e-3	4	NC	1	NC	1
450			min	-.007	2	-.013	4	-.002	1	-1.879e-4	1	8694.172	4	852.374	4
451		17	max	.009	3	-.003	15	.13	4	7.505e-3	4	NC	1	NC	1
452			min	-.008	2	-.009	4	-.003	1	-2.002e-4	1	NC	1	783.89	4
453		18	max	.009	3	-.002	15	.142	4	8.002e-3	4	NC	1	NC	1
454			min	-.008	2	-.005	4	-.004	1	-2.126e-4	1	NC	1	717.75	4
455		19	max	.01	3	0	10	.155	4	8.499e-3	4	NC	1	NC	1
456			min	-.009	2	-.002	3	-.005	1	-2.249e-4	1	NC	1	654.153	4
457	M12	1	max	.002	1	.009	2	.005	1	4.786e-4	5	NC	1	NC	2
458			min	0	3	-.01	3	-.155	4	-8.861e-5	1	NC	1	159.609	4
459		2	max	.002	1	.008	2	.004	1	4.786e-4	5	NC	1	NC	2
460			min	0	3	-.01	3	-.143	4	-8.861e-5	1	NC	1	173.391	4
461		3	max	.002	1	.008	2	.004	1	4.786e-4	5	NC	1	NC	2
462			min	0	3	-.009	3	-.131	4	-8.861e-5	1	NC	1	189.803	4
463		4	max	.002	1	.007	2	.003	1	4.786e-4	5	NC	1	NC	2
464			min	0	3	-.009	3	-.118	4	-8.861e-5	1	NC	1	209.524	4
465		5	max	.002	1	.007	2	.003	1	4.786e-4	5	NC	1	NC	2
466			min	0	3	-.008	3	-.106	4	-8.861e-5	1	NC	1	233.483	4
467		6	max	.001	1	.006	2	.003	1	4.786e-4	5	NC	1	NC	2
468			min	0	3	-.007	3	-.094	4	-8.861e-5	1	NC	1	262.962	4
469		7	max	.001	1	.006	2	.002	1	4.786e-4	5	NC	1	NC	1
470			min	0	3	-.007	3	-.083	4	-8.861e-5	1	NC	1	299.784	4
471		8	max	.001	1	.005	2	.002	1	4.786e-4	5	NC	1	NC	1
472			min	0	3	-.006	3	-.072	4	-8.861e-5	1	NC	1	346.606	4
473		9	max	.001	1	.005	2	.002	1	4.786e-4	5	NC	1	NC	1
474			min	0	3	-.006	3	-.061	4	-8.861e-5	1	NC	1	407.43	4
475		10	max	.001	1	.004	2	.001	1	4.786e-4	5	NC	1	NC	1
476			min	0	3	-.005	3	-.051	4	-8.861e-5	1	NC	1	488.508	4
477		11	max	0	1	.004	2	.001	1	4.786e-4	5	NC	1	NC	1
478			min	0	3	-.005	3	-.041	4	-8.861e-5	1	NC	1	600.069	4
479		12	max	0	1	.003	2	0	1	4.786e-4	5	NC	1	NC	1
480			min	0	3	-.004	3	-.033	4	-8.861e-5	1	NC	1	759.799	4
481		13	max	0	1	.003	2	0	1	4.786e-4	5	NC	1	NC	1
482			min	0	3	-.003	3	-.025	4	-8.861e-5	1	NC	1	1000.523	4
483		14	max	0	1	.002	2	0	1	4.786e-4	5	NC	1	NC	1
484			min	0	3	-.003	3	-.018	4	-8.861e-5	1	NC	1	1388.999	4
485		15	max	0	1	.002	2	0	1	4.786e-4	5	NC	1	NC	1
486			min	0	3	-.002	3	-.012	4	-8.861e-5	1	NC	1	2079.066	4
487		16	max	0	1	.001	2	0	1	4.786e-4	5	NC	1	NC	1
488			min	0	3	-.002	3	-.007	4	-8.861e-5	1	NC	1	3495.817	4



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Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	4.786e-4	5	NC	1	NC	1
490			min	0	3	-.001	3	-.003	4	-8.861e-5	1	NC	1	7214.458	4
491		18	max	0	1	0	2	0	1	4.786e-4	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-8.861e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	4.786e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-8.861e-5	1	NC	1	NC	1
495	M1	1	max	.013	3	.235	2	.548	4	4.632e-3	2	NC	1	NC	1
496			min	-.008	2	-.082	3	0	10	-1.324e-2	3	NC	1	NC	1
497		2	max	.013	3	.113	2	.533	4	5.159e-3	4	NC	5	NC	1
498			min	-.008	2	-.038	3	-.003	1	-6.575e-3	3	1115.775	2	NC	1
499		3	max	.013	3	.02	3	.516	4	9.568e-3	4	NC	5	NC	1
500			min	-.008	2	-.015	2	-.005	1	-9.283e-5	3	542.263	2	7380.199	5
501		4	max	.012	3	.103	3	.499	4	8.175e-3	4	NC	5	NC	1
502			min	-.008	2	-.156	2	-.004	1	-3.557e-3	3	346.948	2	5431.805	5
503		5	max	.012	3	.202	3	.48	4	6.807e-3	2	NC	15	NC	1
504			min	-.008	2	-.3	2	-.003	1	-7.022e-3	3	253.211	2	4446.406	5
505		6	max	.012	3	.306	3	.461	4	1.02e-2	2	9661.374	15	NC	1
506			min	-.008	2	-.439	2	-.001	1	-1.049e-2	3	201.159	2	3829.621	5
507		7	max	.012	3	.405	3	.442	4	1.359e-2	2	8191.852	15	NC	1
508			min	-.008	2	-.561	2	0	3	-1.395e-2	3	170.234	2	3362.128	4
509		8	max	.011	3	.486	3	.423	4	1.699e-2	2	7319.24	15	NC	1
510			min	-.007	2	-.658	2	0	10	-1.742e-2	3	151.862	2	2977.899	4
511		9	max	.011	3	.538	3	.403	4	1.894e-2	2	6861.043	15	NC	1
512			min	-.007	2	-.719	2	0	1	-1.809e-2	3	142.261	2	2685.413	4
513		10	max	.011	3	.558	3	.381	4	1.992e-2	2	6720.479	15	NC	1
514			min	-.007	2	-.739	2	0	10	-1.689e-2	3	139.461	2	2565.545	4
515		11	max	.011	3	.545	3	.357	4	2.09e-2	2	6860.446	15	NC	1
516			min	-.007	2	-.718	2	0	10	-1.57e-2	3	142.811	2	2560.39	4
517		12	max	.01	3	.5	3	.331	4	1.991e-2	2	7317.827	15	NC	1
518			min	-.007	2	-.655	2	0	1	-1.387e-2	3	153.44	2	2658.554	4
519		13	max	.01	3	.427	3	.299	4	1.596e-2	2	8189.132	15	NC	1
520			min	-.007	2	-.553	2	0	1	-1.11e-2	3	173.89	2	3081.607	4
521		14	max	.01	3	.333	3	.264	4	1.201e-2	2	9656.478	15	NC	1
522			min	-.007	2	-.426	2	0	12	-8.323e-3	3	208.698	2	4089.563	4
523		15	max	.009	3	.227	3	.228	4	8.061e-3	2	NC	15	NC	1
524			min	-.007	2	-.285	2	0	10	-5.549e-3	3	268.239	2	6534.521	4
525		16	max	.009	3	.116	3	.192	4	6.956e-3	4	NC	5	NC	1
526			min	-.007	2	-.142	2	0	10	-2.775e-3	3	377.568	2	NC	1
527		17	max	.009	3	.007	3	.159	4	8.073e-3	4	NC	5	NC	1
528			min	-.006	2	-.008	2	0	10	-1.261e-6	3	609.076	2	NC	1
529		18	max	.009	3	.106	2	.132	4	4.389e-3	4	NC	5	NC	1
530			min	-.006	2	-.093	3	0	10	-1.377e-3	3	1282.443	2	NC	1
531		19	max	.009	3	.209	2	.11	4	8.715e-3	2	NC	1	NC	1
532			min	-.006	2	-.187	3	0	1	-2.828e-3	3	NC	1	NC	1
533	M5	1	max	.036	3	.315	2	.548	4	0	1	NC	1	NC	1
534			min	-.025	2	.006	15	0	1	-1.682e-5	4	NC	1	NC	1
535		2	max	.036	3	.15	2	.537	4	4.89e-3	4	NC	5	NC	1
536			min	-.025	2	.003	15	0	1	0	1	832.258	2	NC	1
537		3	max	.036	3	.056	3	.521	4	9.679e-3	4	NC	5	NC	1
538			min	-.025	2	-.041	2	0	1	0	1	385.482	2	6065.934	4
539		4	max	.035	3	.182	3	.503	4	7.885e-3	4	NC	15	NC	1
540			min	-.025	2	-.277	2	0	1	0	1	231.418	2	4769.726	4
541		5	max	.034	3	.365	3	.483	4	6.091e-3	4	8521.585	15	NC	1
542			min	-.024	2	-.538	2	0	1	0	1	160.269	2	4157.199	4
543		6	max	.033	3	.576	3	.462	4	4.298e-3	4	6501.564	15	NC	1
544			min	-.024	2	-.801	2	0	1	0	1	122.403	2	3766.562	4
545		7	max	.032	3	.784	3	.441	4	2.504e-3	4	5346.219	15	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.009	3	.007	3	.156	4	7.891e-3	4	NC	5	NC	1
604		min	-.006	2	-.008	2	-.005	1	-3.161e-4	1	609.076	2	NC	1
605	18	max	.009	3	.106	2	.129	4	3.909e-3	5	NC	5	NC	1
606		min	-.006	2	-.093	3	-.003	1	-4.365e-3	2	1282.443	2	NC	1
607	19	max	.009	3	.209	2	.11	4	2.828e-3	3	NC	1	NC	1
608		min	-.006	2	-.187	3	0	10	-8.715e-3	2	NC	1	NC	1



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Software
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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

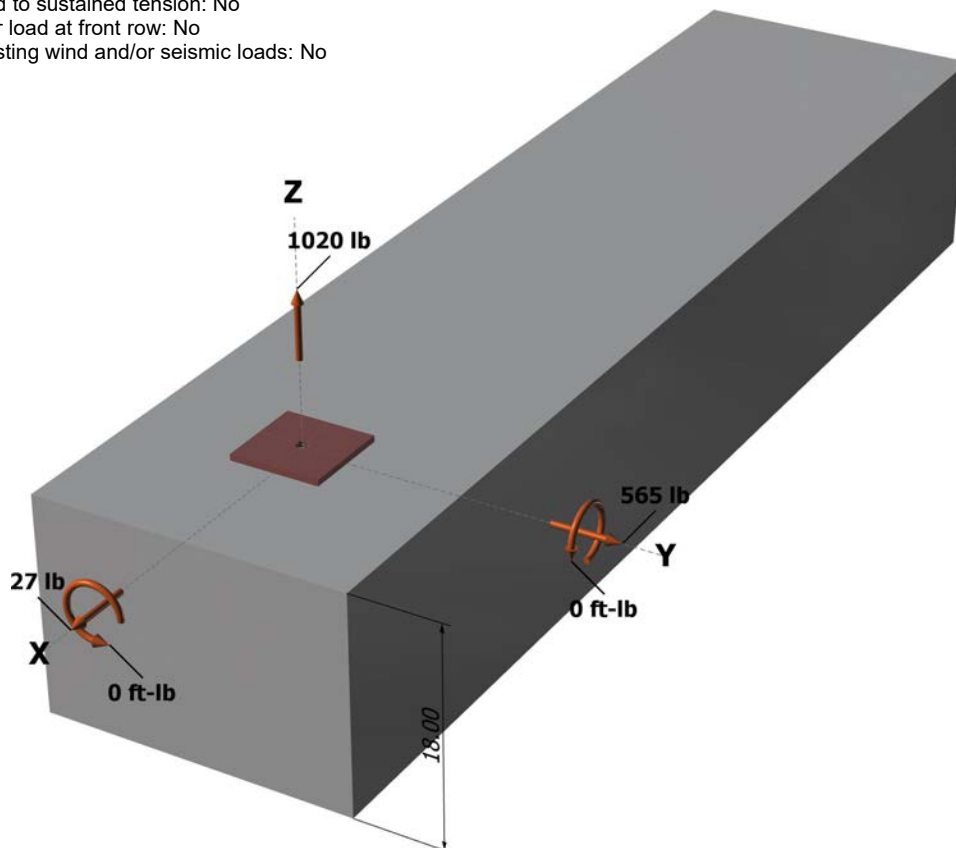
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
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E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.6025.0

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Address:			
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbv} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbv} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
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E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Software
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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

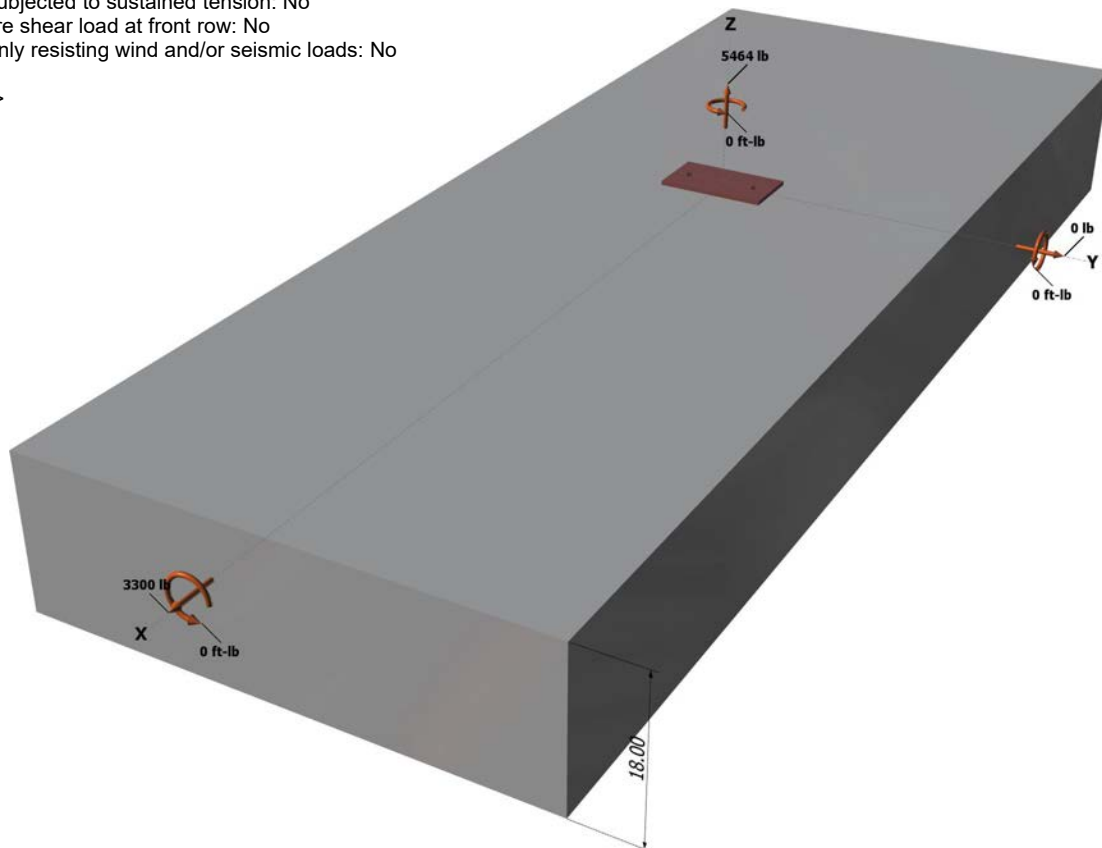
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

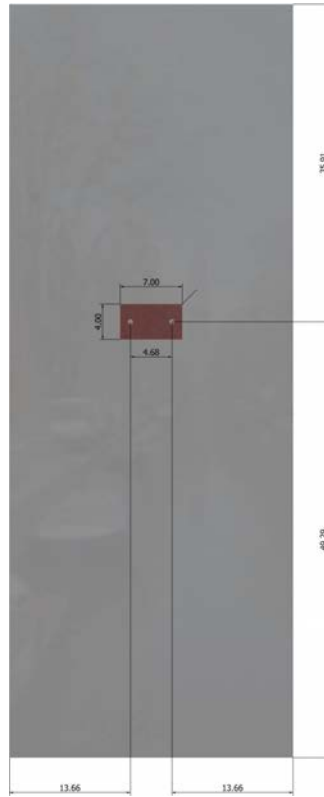
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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

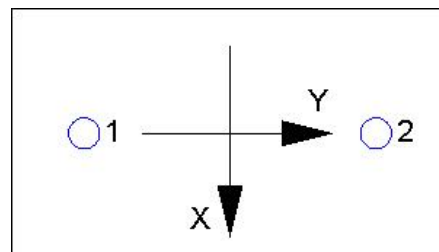
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr,short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.