

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+}$ TOP =	1.100	(Pressure)
$C_{f+}$ BOTTOM =	1.700	
$C_{f-}$ TOP, OUTER PURLIN =	-2.500	
$C_{f-}$ TOP, INNER PURLIN =	-1.900	(Suction)
$C_{f-}$ BOTTOM =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	87 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.863 k-ft
$M_z$ =	0.000 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>67%</b>

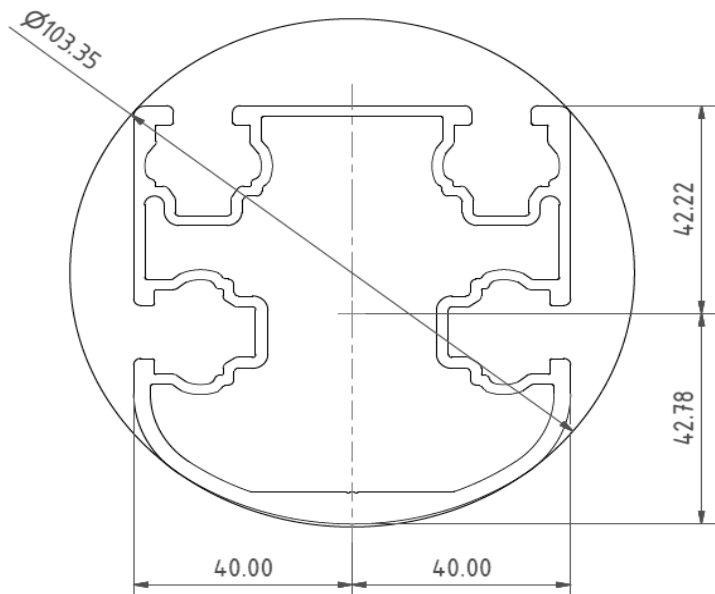


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

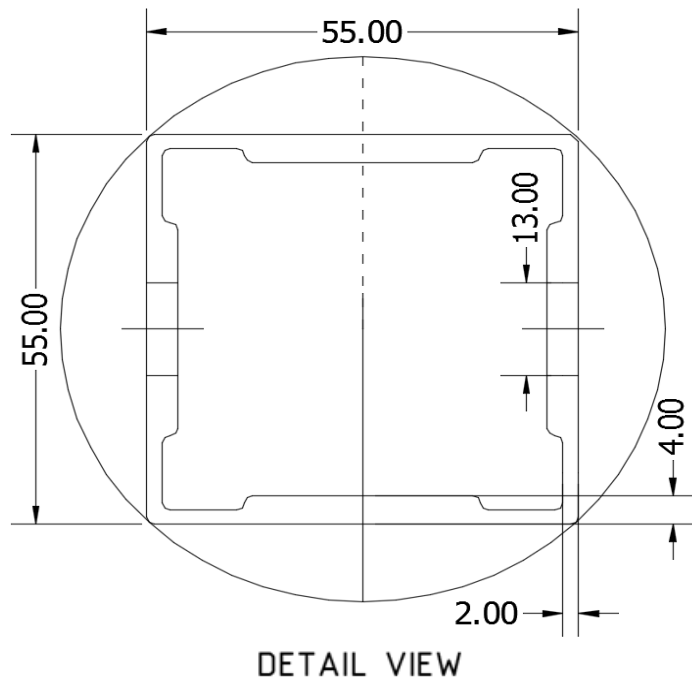
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.378 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.942 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>100%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

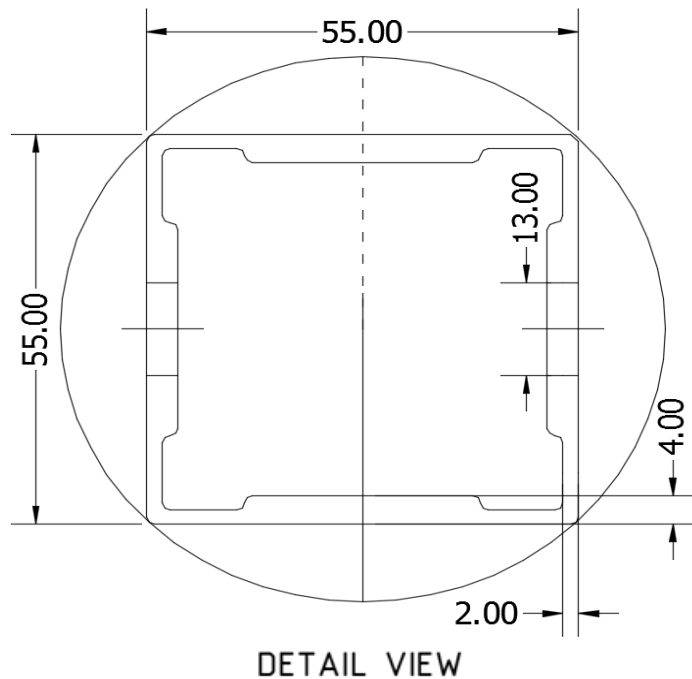
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.465 k-ft
$P_n$ =	0.513 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>35%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.394 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>41%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	69.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.82 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.340 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.629 k
Utilization =	<u>32%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

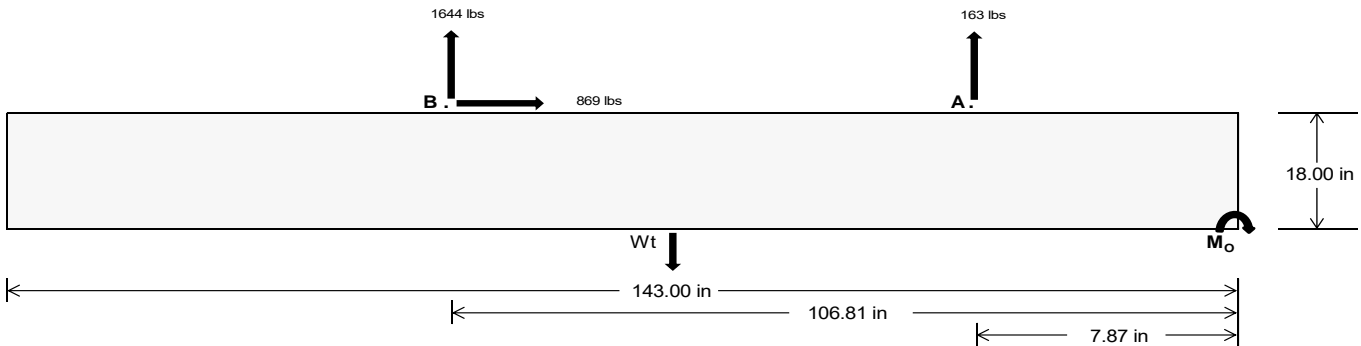
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>686.61</u>	<u>6847.58</u> k
Compressive Load =		<u>3665.22</u>	<u>5100.07</u> k
Lateral Load =		<u>317.04</u>	<u>3614.08</u> k
Moment (Weak Axis) =		<u>0.63</u>	<u>0.27</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 192538.8$  in-lbs  
Resisting Force Required = 2692.85 lbs  
S.F. = 1.67  
Weight Required = 4488.08 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Sliding

Force = 869.00 lbs  
Friction = 0.4  
Weight Required = 2172.50 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 869.00 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1142 lbs	1142 lbs	1142 lbs	1142 lbs	1526 lbs	1526 lbs	1526 lbs	1526 lbs	1894 lbs	1894 lbs	1894 lbs	1894 lbs	-325 lbs	-325 lbs	-325 lbs	-325 lbs
$F_B$	1172 lbs	1172 lbs	1172 lbs	1172 lbs	2207 lbs	2207 lbs	2207 lbs	2207 lbs	2425 lbs	2425 lbs	2425 lbs	2425 lbs	-3288 lbs	-3288 lbs	-3288 lbs	-3288 lbs
$F_V$	128 lbs	128 lbs	128 lbs	128 lbs	1548 lbs	1548 lbs	1548 lbs	1548 lbs	1247 lbs	1247 lbs	1247 lbs	1247 lbs	-1738 lbs	-1738 lbs	-1738 lbs	-1738 lbs
$P_{total}$	9874 lbs	10090 lbs	10306 lbs	10522 lbs	11292 lbs	11508 lbs	11724 lbs	11940 lbs	11879 lbs	12095 lbs	12311 lbs	12527 lbs	922 lbs	1052 lbs	1182 lbs	1311 lbs
$M$	2800 lbs-ft	2800 lbs-ft	2800 lbs-ft	2800 lbs-ft	3917 lbs-ft	3917 lbs-ft	3917 lbs-ft	3917 lbs-ft	4774 lbs-ft	4774 lbs-ft	4774 lbs-ft	4774 lbs-ft	5346 lbs-ft	5346 lbs-ft	5346 lbs-ft	5346 lbs-ft
$e$	0.28 ft	0.28 ft	0.27 ft	0.27 ft	0.35 ft	0.34 ft	0.33 ft	0.33 ft	0.40 ft	0.39 ft	0.39 ft	0.39 ft	5.80 ft	5.08 ft	4.52 ft	4.08 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	243.5 psf	242.8 psf	242.1 psf	241.5 psf	268.1 psf	266.7 psf	265.4 psf	264.2 psf	272.6 psf	271.1 psf	269.6 psf	268.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	324.6 psf	321.7 psf	318.8 psf	316.2 psf	381.6 psf	377.1 psf	372.8 psf	368.7 psf	410.9 psf	405.5 psf	400.5 psf	395.6 psf	1297.4 psf	266.7 psf	178.2 psf	146.7 psf

Maximum Bearing Pressure = 1297 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

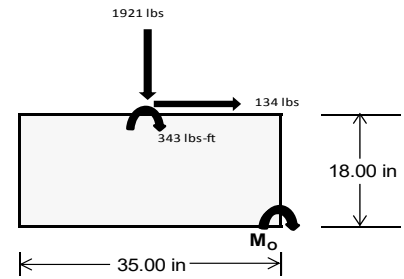
### Overturning Check

$M_o = 2258.1 \text{ ft-lbs}$   
 Resisting Force Required = 1548.44 lbs  
 S.F. = 1.67  
 Weight Required = 2580.74 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	275 lbs	535 lbs	173 lbs	747 lbs	1921 lbs	668 lbs	116 lbs	157 lbs	15 lbs
$F_v$	186 lbs	181 lbs	189 lbs	137 lbs	134 lbs	146 lbs	186 lbs	182 lbs	188 lbs
$P_{total}$	9634 lbs	9894 lbs	9532 lbs	9656 lbs	10830 lbs	9577 lbs	2853 lbs	2893 lbs	2751 lbs
$M$	732 lbs-ft	719 lbs-ft	739 lbs-ft	545 lbs-ft	544 lbs-ft	576 lbs-ft	730 lbs-ft	717 lbs-ft	733 lbs-ft
$e$	0.08 ft	0.07 ft	0.08 ft	0.06 ft	0.05 ft	0.06 ft	0.26 ft	0.25 ft	0.27 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	233.9 psf	242.1 psf	230.5 psf	245.5 psf	279.4 psf	241.4 psf	38.9 psf	40.8 psf	35.8 psf
$f_{max}$	320.5 psf	327.2 psf	318.0 psf	310.1 psf	343.8 psf	309.7 psf	125.3 psf	125.7 psf	122.6 psf



Maximum Bearing Pressure = 344 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 35in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	1.004 k
Allowable Uplift =	1.214 k
Utilization =	<u>83%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.604 k
Allowable Uplift =	4.357 k
Utilization =	<u>60%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.819 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

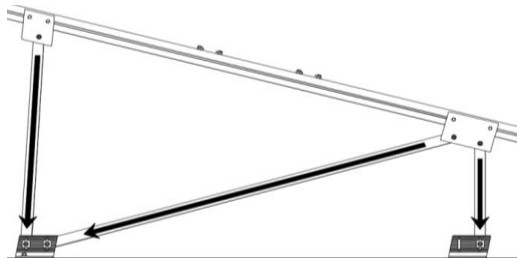
#### Rear Strut

Maximum Axial Load =	4.662 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>63%</u>

#### Diagonal Strut

Maximum Axial Load =	2.572 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>35%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	56.48 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.130 in
	<u>0.595 ≤ 1.13. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 87 \text{ in}$$

$$J = 0.432$$

$$240.683$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 87$$

$$J = 0.432$$

$$153.06$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.80 \text{ in} \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.8 \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.7

$$\begin{aligned} \lambda &= 1.61471 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.80606 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 10.8205 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 10.82 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 11.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-55.176	-55.176	0	0
2	M14	Y	-55.176	-55.176	0	0
3	M15	Y	-55.176	-55.176	0	0
4	M16	Y	-55.176	-55.176	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-81.596	-81.596	0	0
2	M14	y	-81.596	-81.596	0	0
3	M15	y	-126.102	-126.102	0	0
4	M16	y	-126.102	-126.102	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	185.444	185.444	0	0
2	M14	y	140.938	140.938	0	0
3	M15	y	74.178	74.178	0	0
4	M16	y	74.178	74.178	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\... \PVMMax 72 Cell 2V 25° 120mph 30psf 7.25ft 7-05.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	53.449	4	1311.994	3	166.041	1	.007	3	.215	1	1.195	2
20			min	3.958	10	-773.426	2	-103.162	14	-.014	2	0	3	-1.863	3
21		11	max	51.174	1	638.175	2	-3.134	12	.014	2	.1	4	.626	2
22			min	3.958	10	-1077.55	3	-131.351	1	0	15	-.005	3	-.901	3
23		12	max	51.174	1	502.924	2	-1.713	12	.014	2	.052	4	.167	2
24			min	3.958	10	-843.105	3	-96.66	1	0	15	-.008	3	-.127	3
25		13	max	51.174	1	367.674	2	-.224	3	.014	2	.024	5	.458	3
26			min	3.958	10	-608.66	3	-61.97	1	0	15	-.061	1	-.184	2
27		14	max	51.174	1	232.423	2	1.908	3	.014	2	0	15	.853	3
28			min	2.804	15	-374.216	3	-35.721	4	0	15	-.097	1	-.426	2
29		15	max	51.174	1	97.172	2	7.411	1	.014	2	-.004	12	1.06	3
30			min	-5.26	5	-139.771	3	-27.431	5	0	15	-.105	1	-.559	2
31		16	max	51.174	1	94.674	3	42.102	1	.014	2	-.001	12	1.079	3
32			min	-14.766	5	-38.079	2	-25.268	5	0	15	-.085	1	-.582	2
33		17	max	51.174	1	329.119	3	76.792	1	.014	2	.004	3	.908	3
34			min	-24.272	5	-173.33	2	-23.105	5	0	15	-.073	4	-.497	2
35		18	max	51.174	1	563.563	3	111.483	1	.014	2	.039	1	.548	3
36			min	-33.778	5	-308.581	2	-20.941	5	0	15	-.082	5	-.303	2
37		19	max	51.174	1	798.008	3	146.173	1	.014	2	.143	1	0	2
38			min	-43.284	5	-443.832	2	-18.778	5	0	15	-.098	5	0	3
39	M14	1	max	39.251	4	529.298	2	-8.575	12	.015	3	.223	4	0	1
40			min	2.595	12	-653.472	3	-152.849	1	-.016	2	.014	10	0	3
41		2	max	34.089	1	394.047	2	-7.153	12	.015	3	.154	4	.455	3
42			min	2.595	12	-476.391	3	-118.158	1	-.016	2	.003	10	-.372	2
43		3	max	34.089	1	258.796	2	-5.732	12	.015	3	.094	5	.768	3
44			min	2.595	12	-299.31	3	-83.468	1	-.016	2	-.016	1	-.635	2
45		4	max	34.089	1	123.545	2	-3.888	10	.015	3	.054	5	.937	3
46			min	2.208	15	-122.228	3	-61.124	4	-.016	2	-.069	1	-.789	2
47		5	max	34.089	1	54.853	3	-.025	10	.015	3	.015	5	.964	3
48			min	-6.236	5	-15.532	1	-51.418	4	-.016	2	-.094	1	-.834	2
49		6	max	34.089	1	231.935	3	20.603	1	.015	3	-.005	12	.849	3
50			min	-15.742	5	-146.957	2	-45.075	5	-.016	2	-.092	1	-.77	2
51		7	max	34.089	1	409.016	3	55.294	1	.015	3	-.005	10	.591	3
52			min	-25.248	5	-282.208	2	-42.911	5	-.016	2	-.072	4	-.597	2
53		8	max	34.089	1	586.098	3	89.984	1	.015	3	.003	2	.19	3
54			min	-34.754	5	-417.458	2	-40.748	5	-.016	2	-.094	4	-.315	2
55		9	max	34.089	1	763.179	3	124.675	1	.015	3	.084	1	.1	1
56			min	-44.26	5	-552.709	2	-38.585	5	-.016	2	-.124	5	-.354	3
57		10	max	65.357	4	940.26	3	159.365	1	.015	3	.223	4	.575	2
58			min	2.595	12	-687.96	2	-109.139	14	-.016	2	-.001	3	-1.04	3
59		11	max	55.851	4	552.709	2	-2.796	12	.016	2	.153	4	.1	1
60			min	2.595	12	-763.179	3	-124.675	1	-.015	3	-.005	3	-.354	3
61		12	max	46.345	4	417.458	2	-1.375	12	.016	2	.091	4	.19	3
62			min	2.595	12	-586.098	3	-89.984	1	-.015	3	-.008	3	-.315	2
63		13	max	36.839	4	282.208	2	.289	3	.016	2	.05	5	.591	3
64			min	2.595	12	-409.016	3	-62.188	4	-.015	3	-.061	1	-.597	2
65		14	max	34.089	1	146.957	2	2.421	3	.016	2	.011	5	.849	3
66			min	2.595	12	-231.935	3	-52.482	4	-.015	3	-.092	1	-.77	2
67		15	max	34.089	1	15.532	1	14.087	1	.016	2	-.003	12	.964	3
68			min	2.595	12	-54.853	3	-45.313	5	-.015	3	-.094	1	-.834	2
69		16	max	34.089	1	122.228	3	48.777	1	.016	2	0	3	.937	3
70			min	.389	15	-123.545	2	-43.15	5	-.015	3	-.078	4	-.789	2
71		17	max	34.089	1	299.31	3	83.468	1	.016	2	.006	3	.768	3
72			min	-8.907	5	-258.796	2	-40.987	5	-.015	3	-.1	4	-.635	2
73		18	max	34.089	1	476.391	3	118.158	1	.016	2	.066	1	.455	3
74			min	-18.413	5	-394.047	2	-38.823	5	-.015	3	-.128	5	-.372	2
75		19	max	34.089	1	653.472	3	152.849	1	.016	2	.175	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-27.919	5	-529.298	2	-36.66	5	-.015	3	-.159	5	0	3
77	M15	1	max	76.655	5	723.049	2	-8.378	12	.017	2	.288	4	0	2
78			min	-35.74	1	-364.251	3	-152.859	1	-.012	3	.014	10	0	3
79		2	max	67.149	5	530.435	2	-6.957	12	.017	2	.205	4	.257	3
80			min	-35.74	1	-273.216	3	-118.168	1	-.012	3	.003	10	-.505	2
81		3	max	57.643	5	337.821	2	-5.536	12	.017	2	.131	5	.44	3
82			min	-35.74	1	-182.181	3	-88.364	4	-.012	3	-.016	1	-.855	2
83		4	max	48.137	5	145.207	2	-3.985	10	.017	2	.076	5	.55	3
84			min	-35.74	1	-91.146	3	-78.658	4	-.012	3	-.069	1	-1.049	2
85		5	max	38.631	5	.786	12	-.122	10	.017	2	.023	5	.587	3
86			min	-35.74	1	-47.408	2	-68.952	4	-.012	3	-.094	1	-1.089	2
87		6	max	29.125	5	90.925	3	20.594	1	.017	2	-.005	12	.55	3
88			min	-35.74	1	-240.022	2	-62.57	5	-.012	3	-.092	1	-.973	2
89		7	max	19.619	5	181.96	3	55.284	1	.017	2	-.005	10	.441	3
90			min	-35.74	1	-432.636	2	-60.407	5	-.012	3	-.092	4	-.702	2
91		8	max	10.113	5	272.995	3	89.975	1	.017	2	.003	2	.257	3
92			min	-35.74	1	-625.25	2	-58.244	5	-.012	3	-.128	4	-.276	2
93		9	max	.607	5	364.03	3	124.665	1	.017	2	.084	1	.306	2
94			min	-35.74	1	-817.864	2	-56.08	5	-.012	3	-.171	5	-.005	12
95		10	max	-2.738	10	455.066	3	159.356	1	.017	2	.285	4	1.042	2
96			min	-35.74	1	-1010.478	2	-118.066	14	-.012	3	0	3	-.329	3
97		11	max	-2.738	10	817.864	2	-2.992	12	.012	3	.201	4	.306	2
98			min	-35.74	1	-364.03	3	-124.665	1	-.017	2	-.005	3	-.005	12
99		12	max	-2.738	10	625.25	2	-1.571	12	.012	3	.125	4	.257	3
100			min	-35.74	1	-272.995	3	-89.975	1	-.017	2	-.007	3	-.276	2
101		13	max	-2.738	10	432.636	2	-.024	3	.012	3	.07	5	.441	3
102			min	-35.74	1	-181.96	3	-79.757	4	-.017	2	-.061	1	-.702	2
103		14	max	-2.738	10	240.022	2	2.108	3	.012	3	.016	5	.55	3
104			min	-42.864	4	-90.925	3	-70.051	4	-.017	2	-.092	1	-.973	2
105		15	max	-2.738	10	47.408	2	14.097	1	.012	3	-.003	12	.587	3
106			min	-52.37	4	-.786	12	-62.81	5	-.017	2	-.094	1	-1.089	2
107		16	max	-2.738	10	91.146	3	48.787	1	.012	3	0	3	.55	3
108			min	-61.876	4	-145.207	2	-60.647	5	-.017	2	-.101	4	-1.049	2
109		17	max	-2.738	10	182.181	3	83.478	1	.012	3	.006	3	.44	3
110			min	-71.382	4	-337.821	2	-58.484	5	-.017	2	-.138	4	-.855	2
111		18	max	-2.738	10	273.216	3	118.168	1	.012	3	.065	1	.257	3
112			min	-80.888	4	-530.435	2	-56.321	5	-.017	2	-.179	5	-.505	2
113		19	max	-2.738	10	364.251	3	152.859	1	.012	3	.175	1	0	2
114			min	-90.394	4	-723.049	2	-54.157	5	-.017	2	-.224	5	0	5
115	M16	1	max	71.364	5	641.959	2	-7.621	12	.008	1	.201	4	0	2
116			min	-57.618	1	-294.269	3	-146.803	1	-.013	3	.011	12	0	3
117		2	max	61.858	5	449.345	2	-6.2	12	.008	1	.138	4	.2	3
118			min	-57.618	1	-203.234	3	-112.112	1	-.013	3	.002	10	-.44	2
119		3	max	52.352	5	256.731	2	-4.778	12	.008	1	.089	5	.327	3
120			min	-57.618	1	-112.199	3	-77.422	1	-.013	3	-.035	1	-.724	2
121		4	max	42.846	5	64.116	2	-3.357	12	.008	1	.052	5	.381	3
122			min	-57.618	1	-21.164	3	-54.101	4	-.013	3	-.084	1	-.853	2
123		5	max	33.34	5	69.872	3	.253	10	.008	1	.018	5	.362	3
124			min	-57.618	1	-128.498	2	-44.395	4	-.013	3	-.104	1	-.827	2
125		6	max	23.834	5	160.907	3	26.649	1	.008	1	-.005	12	.269	3
126			min	-57.618	1	-321.112	2	-39.686	5	-.013	3	-.097	1	-.646	2
127		7	max	14.328	5	251.942	3	61.34	1	.008	1	-.005	12	.102	3
128			min	-57.618	1	-513.726	2	-37.523	5	-.013	3	-.061	1	-.31	2
129		8	max	4.822	5	342.977	3	96.03	1	.008	1	.004	2	.182	2
130			min	-57.618	1	-706.34	2	-35.359	5	-.013	3	-.077	4	-.137	3
131		9	max	-3.107	15	434.012	3	130.721	1	.008	1	.093	1	.828	2
132			min	-57.618	1	-898.954	2	-33.196	5	-.013	3	-.103	5	-.45	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-4.59	12	525.048	3	165.411	1	.008	1	.213	1	1.63	2
134		min	-57.618	1	-1091.568	2	-109.799	14	-.013	3	.003	12	-.837	3
135	11	max	-1.617	15	898.954	2	-3.75	12	.013	3	.136	4	.828	2
136		min	-57.618	1	-434.012	3	-130.721	1	-.008	1	-.002	3	-.45	3
137	12	max	-4.59	12	706.34	2	-2.328	12	.013	3	.077	4	.182	2
138		min	-57.618	1	-342.977	3	-96.03	1	-.008	1	-.006	3	-.137	3
139	13	max	-4.59	12	513.726	2	-.907	12	.013	3	.038	5	.102	3
140		min	-57.618	1	-251.942	3	-61.34	1	-.008	1	-.061	1	-.31	2
141	14	max	-4.59	12	321.112	2	.923	3	.013	3	.003	5	.269	3
142		min	-57.618	1	-160.907	3	-49.052	4	-.008	1	-.097	1	-.646	2
143	15	max	-4.59	12	128.498	2	8.041	1	.013	3	-.004	12	.362	3
144		min	-57.618	1	-69.872	3	-40.739	5	-.008	1	-.104	1	-.827	2
145	16	max	-4.59	12	21.164	3	42.731	1	.013	3	-.002	12	.381	3
146		min	-63.577	4	-64.116	2	-38.576	5	-.008	1	-.084	1	-.853	2
147	17	max	-4.59	12	112.199	3	77.422	1	.013	3	.002	3	.327	3
148		min	-73.083	4	-256.731	2	-36.413	5	-.008	1	-.101	4	-.724	2
149	18	max	-4.59	12	203.234	3	112.112	1	.013	3	.041	1	.2	3
150		min	-82.589	4	-449.345	2	-34.249	5	-.008	1	-.121	5	-.44	2
151	19	max	-4.59	12	294.269	3	146.803	1	.013	3	.145	1	0	2
152		min	-92.094	4	-641.959	2	-32.086	5	-.008	1	-.148	5	0	5
153	M2	1	max	1110.267	2	2.071	4	.406	1	0	0	3	0	1
154		min	-1508.103	3	.507	15	-32.006	4	0	4	0	2	0	1
155	2	max	1110.741	2	2.034	4	.406	1	0	3	0	1	0	15
156		min	-1507.748	3	.498	15	-32.417	4	0	4	-.01	4	0	4
157	3	max	1111.215	2	1.997	4	.406	1	0	3	0	1	0	15
158		min	-1507.393	3	.49	15	-32.828	4	0	4	-.021	4	-.001	4
159	4	max	1111.689	2	1.96	4	.406	1	0	3	0	1	0	15
160		min	-1507.037	3	.481	15	-33.24	4	0	4	-.031	4	-.002	4
161	5	max	1112.162	2	1.923	4	.406	1	0	3	0	1	0	15
162		min	-1506.682	3	.472	15	-33.651	4	0	4	-.042	4	-.003	4
163	6	max	1112.636	2	1.886	4	.406	1	0	3	0	1	0	15
164		min	-1506.327	3	.463	15	-34.062	4	0	4	-.053	4	-.003	4
165	7	max	1113.11	2	1.849	4	.406	1	0	3	0	1	0	15
166		min	-1505.971	3	.455	15	-34.474	4	0	4	-.064	4	-.004	4
167	8	max	1113.583	2	1.812	4	.406	1	0	3	0	1	-.001	15
168		min	-1505.616	3	.446	15	-34.885	4	0	4	-.075	4	-.004	4
169	9	max	1114.057	2	1.775	4	.406	1	0	3	.001	1	-.001	15
170		min	-1505.261	3	.437	15	-35.296	4	0	4	-.086	4	-.005	4
171	10	max	1114.531	2	1.738	4	.406	1	0	3	.001	1	-.001	15
172		min	-1504.906	3	.429	15	-35.708	4	0	4	-.097	4	-.005	4
173	11	max	1115.005	2	1.701	4	.406	1	0	3	.001	1	-.001	15
174		min	-1504.55	3	.419	12	-36.119	4	0	4	-.109	4	-.006	4
175	12	max	1115.478	2	1.664	4	.406	1	0	3	.001	1	-.002	15
176		min	-1504.195	3	.405	12	-36.53	4	0	4	-.121	4	-.007	4
177	13	max	1115.952	2	1.627	4	.406	1	0	3	.002	1	-.002	15
178		min	-1503.84	3	.391	12	-36.942	4	0	4	-.132	4	-.007	4
179	14	max	1116.426	2	1.59	4	.406	1	0	3	.002	1	-.002	15
180		min	-1503.484	3	.376	12	-37.353	4	0	4	-.144	4	-.008	4
181	15	max	1116.9	2	1.553	4	.406	1	0	3	.002	1	-.002	15
182		min	-1503.129	3	.362	12	-37.764	4	0	4	-.156	4	-.008	4
183	16	max	1117.373	2	1.516	4	.406	1	0	3	.002	1	-.002	15
184		min	-1502.774	3	.347	12	-38.176	4	0	4	-.168	4	-.009	4
185	17	max	1117.847	2	1.479	4	.406	1	0	3	.002	1	-.002	15
186		min	-1502.418	3	.333	12	-38.587	4	0	4	-.181	4	-.009	4
187	18	max	1118.321	2	1.442	4	.406	1	0	3	.002	1	-.002	15
188		min	-1502.063	3	.318	12	-38.998	4	0	4	-.193	4	-.01	4
189	19	max	1118.795	2	1.405	4	.406	1	0	3	.002	1	-.002	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1501.708	3	.304	12	-39.41	4	0	4	-.206	4	-.01	4
191	M3	1	max	730.606	2	9.026	4	.19	1	0	0	1	.01	4
192		min	-870.657	3	2.135	15	-.646	5	0	4	-.013	4	.002	15
193		2	max	730.436	2	8.154	4	.19	1	0	0	1	.006	4
194		min	-870.785	3	1.93	15	-.039	5	0	4	-.013	4	0	12
195		3	max	730.266	2	7.282	4	.666	4	0	0	1	.003	2
196		min	-870.912	3	1.725	15	.015	12	0	4	-.013	4	0	3
197		4	max	730.095	2	6.41	4	1.273	4	0	0	1	0	2
198		min	-871.04	3	1.52	15	.015	12	0	4	-.012	4	-.002	3
199		5	max	729.925	2	5.538	4	1.88	4	0	0	1	0	15
200		min	-871.168	3	1.315	15	.015	12	0	4	-.012	5	-.004	3
201		6	max	729.755	2	4.666	4	2.487	4	0	0	1	-.001	15
202		min	-871.296	3	1.11	15	.015	12	0	4	-.011	5	-.006	6
203		7	max	729.584	2	3.794	4	3.094	4	0	0	1	-.002	15
204		min	-871.423	3	.905	15	.015	12	0	4	-.009	5	-.008	6
205		8	max	729.414	2	2.922	4	3.701	4	0	0	1	-.002	15
206		min	-871.551	3	.7	15	.015	12	0	4	-.008	5	-.01	6
207		9	max	729.244	2	2.05	4	4.308	4	0	0	1	-.003	15
208		min	-871.679	3	.495	15	.015	12	0	4	-.006	5	-.011	6
209		10	max	729.073	2	1.178	4	4.915	4	0	0	1	-.003	15
210		min	-871.807	3	.29	15	.015	12	0	4	-.004	5	-.012	6
211		11	max	728.903	2	.392	2	5.523	4	0	0	1	-.003	15
212		min	-871.934	3	-.084	3	.015	12	0	4	-.001	5	-.012	6
213		12	max	728.733	2	-.12	15	6.13	4	0	0	4	-.003	15
214		min	-872.062	3	-.594	3	.015	12	0	4	0	12	-.012	6
215		13	max	728.562	2	-.325	15	6.737	4	0	0	4	-.003	15
216		min	-872.19	3	-1.44	6	.015	12	0	4	0	12	-.011	6
217		14	max	728.392	2	-.53	15	7.344	4	0	0	4	-.002	15
218		min	-872.318	3	-2.312	6	.015	12	0	4	0	12	-.011	6
219		15	max	728.222	2	-.735	15	7.951	4	0	0	4	-.002	15
220		min	-872.446	3	-3.184	6	.015	12	0	4	0	12	-.009	6
221		16	max	728.051	2	-.94	15	8.558	4	0	0	4	-.002	15
222		min	-872.573	3	-4.056	6	.015	12	0	4	0	12	-.008	6
223		17	max	727.881	2	-1.144	15	9.165	4	0	0	4	-.001	15
224		min	-872.701	3	-4.928	6	.015	12	0	4	0	12	-.005	6
225		18	max	727.711	2	-1.349	15	9.772	4	0	0	4	0	15
226		min	-872.829	3	-5.8	6	.015	12	0	4	0	12	-.003	6
227		19	max	727.54	2	-1.554	15	10.379	4	0	0	4	0	1
228		min	-872.957	3	-6.672	6	.015	12	0	4	0	12	0	1
229	M4	1	max	1037.494	1	0	1	-.681	12	0	0	4	0	1
230		min	-146.789	3	0	1	-241.956	4	0	1	0	10	0	1
231		2	max	1037.665	1	0	1	-.681	12	0	0	1	0	1
232		min	-146.661	3	0	1	-242.103	4	0	1	-.008	4	0	1
233		3	max	1037.835	1	0	1	-.681	12	0	0	12	0	1
234		min	-146.533	3	0	1	-242.251	4	0	1	-.036	4	0	1
235		4	max	1038.005	1	0	1	-.681	12	0	0	12	0	1
236		min	-146.405	3	0	1	-242.399	4	0	1	-.063	4	0	1
237		5	max	1038.176	1	0	1	-.681	12	0	0	12	0	1
238		min	-146.278	3	0	1	-242.546	4	0	1	-.091	4	0	1
239		6	max	1038.346	1	0	1	-.681	12	0	0	12	0	1
240		min	-146.15	3	0	1	-242.694	4	0	1	-.119	4	0	1
241		7	max	1038.516	1	0	1	-.681	12	0	0	12	0	1
242		min	-146.022	3	0	1	-242.842	4	0	1	-.147	4	0	1
243		8	max	1038.687	1	0	1	-.681	12	0	0	12	0	1
244		min	-145.894	3	0	1	-242.989	4	0	1	-.175	4	0	1
245		9	max	1038.857	1	0	1	-.681	12	0	0	12	0	1
246		min	-145.766	3	0	1	-243.137	4	0	1	-.203	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1039.028	1	0	1	-.681	12	0	1	0	12	0	1
248		min	-145.639	3	0	1	-243.285	4	0	1	-.231	4	0	1
249	11	max	1039.198	1	0	1	-.681	12	0	1	0	12	0	1
250		min	-145.511	3	0	1	-243.432	4	0	1	-.259	4	0	1
251	12	max	1039.368	1	0	1	-.681	12	0	1	0	12	0	1
252		min	-145.383	3	0	1	-243.58	4	0	1	-.287	4	0	1
253	13	max	1039.539	1	0	1	-.681	12	0	1	0	12	0	1
254		min	-145.255	3	0	1	-243.727	4	0	1	-.315	4	0	1
255	14	max	1039.709	1	0	1	-.681	12	0	1	0	12	0	1
256		min	-145.128	3	0	1	-243.875	4	0	1	-.343	4	0	1
257	15	max	1039.879	1	0	1	-.681	12	0	1	0	12	0	1
258		min	-145	3	0	1	-244.023	4	0	1	-.371	4	0	1
259	16	max	1040.05	1	0	1	-.681	12	0	1	-.001	12	0	1
260		min	-144.872	3	0	1	-244.17	4	0	1	-.399	4	0	1
261	17	max	1040.22	1	0	1	-.681	12	0	1	-.001	12	0	1
262		min	-144.744	3	0	1	-244.318	4	0	1	-.427	4	0	1
263	18	max	1040.39	1	0	1	-.681	12	0	1	-.001	12	0	1
264		min	-144.617	3	0	1	-244.466	4	0	1	-.455	4	0	1
265	19	max	1040.561	1	0	1	-.681	12	0	1	-.001	12	0	1
266		min	-144.489	3	0	1	-244.613	4	0	1	-.483	4	0	1
267	M6	1	max	3331.336	2	2.42	2	0	1	0	0	4	0	1
268		min	-4662.288	3	.103	3	-32.35	4	0	4	0	1	0	1
269	2	max	3331.81	2	2.391	2	0	1	0	1	0	1	0	3
270		min	-4661.933	3	.082	3	-32.761	4	0	4	-.01	4	0	2
271	3	max	3332.283	2	2.362	2	0	1	0	1	0	1	0	3
272		min	-4661.577	3	.06	3	-33.173	4	0	4	-.021	4	-.002	2
273	4	max	3332.757	2	2.334	2	0	1	0	1	0	1	0	3
274		min	-4661.222	3	.038	3	-33.584	4	0	4	-.032	4	-.002	2
275	5	max	3333.231	2	2.305	2	0	1	0	1	0	1	0	3
276		min	-4660.867	3	.017	3	-33.995	4	0	4	-.042	4	-.003	2
277	6	max	3333.705	2	2.276	2	0	1	0	1	0	1	0	3
278		min	-4660.512	3	-.005	3	-34.407	4	0	4	-.053	4	-.004	2
279	7	max	3334.178	2	2.247	2	0	1	0	1	0	1	0	3
280		min	-4660.156	3	-.027	3	-34.818	4	0	4	-.064	4	-.004	2
281	8	max	3334.652	2	2.218	2	0	1	0	1	0	1	0	3
282		min	-4659.801	3	-.048	3	-35.229	4	0	4	-.076	4	-.005	2
283	9	max	3335.126	2	2.189	2	0	1	0	1	0	1	0	3
284		min	-4659.446	3	-.07	3	-35.641	4	0	4	-.087	4	-.006	2
285	10	max	3335.6	2	2.16	2	0	1	0	1	0	1	0	3
286		min	-4659.09	3	-.092	3	-36.052	4	0	4	-.098	4	-.007	2
287	11	max	3336.073	2	2.132	2	0	1	0	1	0	1	0	3
288		min	-4658.735	3	-.113	3	-36.463	4	0	4	-.11	4	-.007	2
289	12	max	3336.547	2	2.103	2	0	1	0	1	0	1	0	3
290		min	-4658.38	3	-.135	3	-36.875	4	0	4	-.122	4	-.008	2
291	13	max	3337.021	2	2.074	2	0	1	0	1	0	1	0	3
292		min	-4658.024	3	-.157	3	-37.286	4	0	4	-.134	4	-.009	2
293	14	max	3337.494	2	2.045	2	0	1	0	1	0	1	0	3
294		min	-4657.669	3	-.178	3	-37.697	4	0	4	-.146	4	-.009	2
295	15	max	3337.968	2	2.016	2	0	1	0	1	0	1	0	3
296		min	-4657.314	3	-.2	3	-38.109	4	0	4	-.158	4	-.01	2
297	16	max	3338.442	2	1.987	2	0	1	0	1	0	1	0	3
298		min	-4656.959	3	-.222	3	-38.52	4	0	4	-.17	4	-.011	2
299	17	max	3338.916	2	1.958	2	0	1	0	1	0	1	0	3
300		min	-4656.603	3	-.243	3	-38.931	4	0	4	-.182	4	-.011	2
301	18	max	3339.389	2	1.929	2	0	1	0	1	0	1	0	3
302		min	-4656.248	3	-.265	3	-39.343	4	0	4	-.195	4	-.012	2
303	19	max	3339.863	2	1.901	2	0	1	0	1	0	1	0	3



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Nov 23, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-4655.893	3	-.287	3	-39.754	4	0	4	-.208	4	-.012	2
305	M7	1	max	2393.964	2	9.02	6	0	1	0	0	1	.012	2
306		min	-2569.456	3	2.118	15	-.898	5	0	4	-.013	4	0	3
307		2	max	2393.794	2	8.148	6	0	1	0	0	1	.009	2
308		min	-2569.584	3	1.913	15	-.291	5	0	4	-.013	4	-.003	3
309		3	max	2393.624	2	7.276	6	.367	4	0	0	1	.006	2
310		min	-2569.711	3	1.708	15	0	1	0	4	-.013	4	-.004	3
311		4	max	2393.453	2	6.404	6	.974	4	0	0	1	.003	2
312		min	-2569.839	3	1.503	15	0	1	0	4	-.013	4	-.006	3
313		5	max	2393.283	2	5.532	6	1.581	4	0	0	1	0	2
314		min	-2569.967	3	1.298	15	0	1	0	4	-.012	4	-.007	3
315		6	max	2393.113	2	4.66	6	2.188	4	0	0	1	-.001	2
316		min	-2570.095	3	1.093	15	0	1	0	4	-.011	4	-.008	3
317		7	max	2392.942	2	3.788	6	2.795	4	0	0	1	-.002	15
318		min	-2570.222	3	.888	15	0	1	0	4	-.01	4	-.009	3
319		8	max	2392.772	2	2.916	6	3.402	4	0	0	1	-.002	15
320		min	-2570.35	3	.668	12	0	1	0	4	-.009	4	-.01	4
321		9	max	2392.602	2	2.143	2	4.01	4	0	0	1	-.003	15
322		min	-2570.478	3	.329	12	0	1	0	4	-.007	5	-.011	4
323		10	max	2392.431	2	1.464	2	4.617	4	0	0	1	-.003	15
324		min	-2570.606	3	-.062	3	0	1	0	4	-.005	5	-.012	4
325		11	max	2392.261	2	.784	2	5.224	4	0	0	1	-.003	15
326		min	-2570.733	3	-.572	3	0	1	0	4	-.003	5	-.012	4
327		12	max	2392.091	2	.105	2	5.831	4	0	0	14	-.003	15
328		min	-2570.861	3	-1.081	3	0	1	0	4	0	5	-.012	4
329		13	max	2391.92	2	-.342	15	6.438	4	0	.003	4	-.003	15
330		min	-2570.989	3	-1.591	3	0	1	0	4	0	1	-.012	4
331		14	max	2391.75	2	-.547	15	7.045	4	0	.006	4	-.003	15
332		min	-2571.117	3	-2.316	4	0	1	0	4	0	1	-.011	4
333		15	max	2391.58	2	-.752	15	7.652	4	0	.009	4	-.002	15
334		min	-2571.244	3	-3.188	4	0	1	0	4	0	1	-.009	4
335		16	max	2391.409	2	-.957	15	8.259	4	0	.013	4	-.002	15
336		min	-2571.372	3	-4.06	4	0	1	0	4	0	1	-.008	4
337		17	max	2391.239	2	-1.162	15	8.866	4	0	.017	4	-.001	15
338		min	-2571.5	3	-4.933	4	0	1	0	4	0	1	-.005	4
339		18	max	2391.069	2	-1.367	15	9.474	4	0	.022	4	0	15
340		min	-2571.628	3	-5.805	4	0	1	0	4	0	1	-.003	4
341		19	max	2390.898	2	-1.572	15	10.081	4	0	.026	4	0	1
342		min	-2571.755	3	-6.677	4	0	1	0	4	0	1	0	1
343	M8	1	max	2816.332	2	0	1	0	1	0	.018	4	0	1
344		min	-530.462	3	0	1	-232.637	4	0	1	0	1	0	1
345		2	max	2816.503	2	0	1	0	1	0	0	1	0	1
346		min	-530.334	3	0	1	-232.784	4	0	1	-.008	4	0	1
347		3	max	2816.673	2	0	1	0	1	0	0	1	0	1
348		min	-530.206	3	0	1	-232.932	4	0	1	-.035	4	0	1
349		4	max	2816.844	2	0	1	0	1	0	0	1	0	1
350		min	-530.079	3	0	1	-233.08	4	0	1	-.062	4	0	1
351		5	max	2817.014	2	0	1	0	1	0	0	1	0	1
352		min	-529.951	3	0	1	-233.227	4	0	1	-.089	4	0	1
353		6	max	2817.184	2	0	1	0	1	0	0	1	0	1
354		min	-529.823	3	0	1	-233.375	4	0	1	-.116	4	0	1
355		7	max	2817.355	2	0	1	0	1	0	0	1	0	1
356		min	-529.695	3	0	1	-233.523	4	0	1	-.142	4	0	1
357		8	max	2817.525	2	0	1	0	1	0	0	1	0	1
358		min	-529.568	3	0	1	-233.67	4	0	1	-.169	4	0	1
359		9	max	2817.695	2	0	1	0	1	0	0	1	0	1
360		min	-529.44	3	0	1	-233.818	4	0	1	-.196	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2817.866	2	0	1	0	1	0	1	0	1	0	1
362			min	-529.312	3	0	1	-233.966	4	0	1	-.223	4	0	1
363		11	max	2818.036	2	0	1	0	1	0	1	0	1	0	1
364			min	-529.184	3	0	1	-234.113	4	0	1	-.25	4	0	1
365		12	max	2818.206	2	0	1	0	1	0	1	0	1	0	1
366			min	-529.057	3	0	1	-234.261	4	0	1	-.277	4	0	1
367		13	max	2818.377	2	0	1	0	1	0	1	0	1	0	1
368			min	-528.929	3	0	1	-234.408	4	0	1	-.304	4	0	1
369		14	max	2818.547	2	0	1	0	1	0	1	0	1	0	1
370			min	-528.801	3	0	1	-234.556	4	0	1	-.33	4	0	1
371		15	max	2818.717	2	0	1	0	1	0	1	0	1	0	1
372			min	-528.673	3	0	1	-234.704	4	0	1	-.357	4	0	1
373		16	max	2818.888	2	0	1	0	1	0	1	0	1	0	1
374			min	-528.546	3	0	1	-234.851	4	0	1	-.384	4	0	1
375		17	max	2819.058	2	0	1	0	1	0	1	0	1	0	1
376			min	-528.418	3	0	1	-234.999	4	0	1	-.411	4	0	1
377		18	max	2819.228	2	0	1	0	1	0	1	0	1	0	1
378			min	-528.29	3	0	1	-235.147	4	0	1	-.438	4	0	1
379		19	max	2819.399	2	0	1	0	1	0	1	0	1	0	1
380			min	-528.162	3	0	1	-235.294	4	0	1	-.465	4	0	1
381	M10	1	max	1110.267	2	1.98	6	-.032	10	0	1	0	4	0	1
382			min	-1508.103	3	.445	15	-32.239	4	0	5	0	3	0	1
383		2	max	1110.741	2	1.943	6	-.032	10	0	1	0	10	0	15
384			min	-1507.748	3	.437	15	-32.651	4	0	5	-.01	4	0	6
385		3	max	1111.215	2	1.906	6	-.032	10	0	1	0	10	0	15
386			min	-1507.393	3	.428	15	-33.062	4	0	5	-.021	4	-.001	6
387		4	max	1111.689	2	1.869	6	-.032	10	0	1	0	10	0	15
388			min	-1507.037	3	.419	15	-33.473	4	0	5	-.032	4	-.002	6
389		5	max	1112.162	2	1.832	6	-.032	10	0	1	0	10	0	15
390			min	-1506.682	3	.41	15	-33.885	4	0	5	-.042	4	-.002	6
391		6	max	1112.636	2	1.795	6	-.032	10	0	1	0	10	0	15
392			min	-1506.327	3	.402	15	-34.296	4	0	5	-.053	4	-.003	6
393		7	max	1113.11	2	1.758	6	-.032	10	0	1	0	10	0	15
394			min	-1505.971	3	.393	15	-34.707	4	0	5	-.064	4	-.004	6
395		8	max	1113.583	2	1.721	6	-.032	10	0	1	0	10	0	15
396			min	-1505.616	3	.384	15	-35.119	4	0	5	-.075	4	-.004	6
397		9	max	1114.057	2	1.684	6	-.032	10	0	1	0	10	-.001	15
398			min	-1505.261	3	.376	15	-35.53	4	0	5	-.087	4	-.005	6
399		10	max	1114.531	2	1.647	6	-.032	10	0	1	0	10	-.001	15
400			min	-1504.906	3	.367	15	-35.941	4	0	5	-.098	4	-.005	6
401		11	max	1115.005	2	1.61	6	-.032	10	0	1	0	10	-.001	15
402			min	-1504.55	3	.358	15	-36.353	4	0	5	-.11	4	-.006	6
403		12	max	1115.478	2	1.573	6	-.032	10	0	1	0	10	-.001	15
404			min	-1504.195	3	.349	15	-36.764	4	0	5	-.121	4	-.006	6
405		13	max	1115.952	2	1.535	6	-.032	10	0	1	0	10	-.002	15
406			min	-1503.84	3	.341	15	-37.175	4	0	5	-.133	4	-.007	6
407		14	max	1116.426	2	1.498	6	-.032	10	0	1	0	10	-.002	15
408			min	-1503.484	3	.332	15	-37.587	4	0	5	-.145	4	-.007	6
409		15	max	1116.9	2	1.461	6	-.032	10	0	1	0	10	-.002	15
410			min	-1503.129	3	.323	15	-37.998	4	0	5	-.157	4	-.008	6
411		16	max	1117.373	2	1.424	6	-.032	10	0	1	0	10	-.002	15
412			min	-1502.774	3	.315	15	-38.409	4	0	5	-.17	4	-.008	6
413		17	max	1117.847	2	1.387	6	-.032	10	0	1	0	10	-.002	15
414			min	-1502.418	3	.306	15	-38.821	4	0	5	-.182	4	-.009	6
415		18	max	1118.321	2	1.35	6	-.032	10	0	1	0	10	-.002	15
416			min	-1502.063	3	.297	15	-39.232	4	0	5	-.194	4	-.009	6
417		19	max	1118.795	2	1.321	2	-.032	10	0	1	0	10	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1501.708	3	.288	15	-39.643	4	0	5	-.207	4	-.009	6
419	M11	1	max	730.606	2	8.964	6	-.015	12	0	1	0	10	.009	6
420			min	-870.657	3	2.093	15	-.67	5	0	4	-.013	4	.002	15
421		2	max	730.436	2	8.092	6	-.015	12	0	1	0	10	.006	2
422			min	-870.785	3	1.888	15	-.19	1	0	4	-.013	4	0	12
423		3	max	730.266	2	7.22	6	.557	4	0	1	0	10	.003	2
424			min	-870.912	3	1.683	15	-.19	1	0	4	-.013	4	0	3
425		4	max	730.095	2	6.348	6	1.164	4	0	1	0	10	0	2
426			min	-871.04	3	1.478	15	-.19	1	0	4	-.013	4	-.002	3
427		5	max	729.925	2	5.476	6	1.771	4	0	1	0	10	-.001	15
428			min	-871.168	3	1.273	15	-.19	1	0	4	-.012	4	-.004	4
429		6	max	729.755	2	4.604	6	2.378	4	0	1	0	10	-.002	15
430			min	-871.296	3	1.068	15	-.19	1	0	4	-.011	4	-.007	4
431		7	max	729.584	2	3.732	6	2.985	4	0	1	0	10	-.002	15
432			min	-871.423	3	.863	15	-.19	1	0	4	-.01	4	-.009	4
433		8	max	729.414	2	2.86	6	3.593	4	0	1	0	10	-.002	15
434			min	-871.551	3	.659	15	-.19	1	0	4	-.008	4	-.01	4
435		9	max	729.244	2	1.988	6	4.2	4	0	1	0	10	-.003	15
436			min	-871.679	3	.454	15	-.19	1	0	4	-.006	4	-.011	4
437		10	max	729.073	2	1.116	6	4.807	4	0	1	0	10	-.003	15
438			min	-871.807	3	.249	15	-.19	1	0	4	-.004	4	-.012	4
439		11	max	728.903	2	.392	2	5.414	4	0	1	0	12	-.003	15
440			min	-871.934	3	-.084	3	-.19	1	0	4	-.002	4	-.012	4
441		12	max	728.733	2	-.161	15	6.021	4	0	1	.001	5	-.003	15
442			min	-872.062	3	-.63	4	-.19	1	0	4	-.001	1	-.012	4
443		13	max	728.562	2	-.366	15	6.628	4	0	1	.004	5	-.003	15
444			min	-872.19	3	-1.502	4	-.19	1	0	4	-.001	1	-.012	4
445		14	max	728.392	2	-.571	15	7.235	4	0	1	.007	5	-.003	15
446			min	-872.318	3	-2.374	4	-.19	1	0	4	-.001	1	-.011	4
447		15	max	728.222	2	-.776	15	7.842	4	0	1	.011	5	-.002	15
448			min	-872.446	3	-3.246	4	-.19	1	0	4	-.001	1	-.009	4
449		16	max	728.051	2	-.981	15	8.45	4	0	1	.015	5	-.002	15
450			min	-872.573	3	-4.118	4	-.19	1	0	4	-.001	1	-.008	4
451		17	max	727.881	2	-1.186	15	9.057	4	0	1	.019	5	-.001	15
452			min	-872.701	3	-4.99	4	-.19	1	0	4	-.002	1	-.006	4
453		18	max	727.711	2	-1.391	15	9.664	4	0	1	.023	5	0	15
454			min	-872.829	3	-5.862	4	-.19	1	0	4	-.002	1	-.003	4
455		19	max	727.54	2	-1.596	15	10.271	4	0	1	.028	5	0	1
456			min	-872.957	3	-6.734	4	-.19	1	0	4	-.002	1	0	1
457	M12	1	max	1037.494	1	0	1	8.87	1	0	1	.019	5	0	1
458			min	-146.789	3	0	1	-236.844	4	0	1	-.001	1	0	1
459		2	max	1037.665	1	0	1	8.87	1	0	1	0	10	0	1
460			min	-146.661	3	0	1	-236.991	4	0	1	-.008	4	0	1
461		3	max	1037.835	1	0	1	8.87	1	0	1	0	1	0	1
462			min	-146.533	3	0	1	-237.139	4	0	1	-.035	4	0	1
463		4	max	1038.005	1	0	1	8.87	1	0	1	.002	1	0	1
464			min	-146.405	3	0	1	-237.286	4	0	1	-.062	4	0	1
465		5	max	1038.176	1	0	1	8.87	1	0	1	.003	1	0	1
466			min	-146.278	3	0	1	-237.434	4	0	1	-.09	4	0	1
467		6	max	1038.346	1	0	1	8.87	1	0	1	.004	1	0	1
468			min	-146.15	3	0	1	-237.582	4	0	1	-.117	4	0	1
469		7	max	1038.516	1	0	1	8.87	1	0	1	.005	1	0	1
470			min	-146.022	3	0	1	-237.729	4	0	1	-.144	4	0	1
471		8	max	1038.687	1	0	1	8.87	1	0	1	.006	1	0	1
472			min	-145.894	3	0	1	-237.877	4	0	1	-.171	4	0	1
473		9	max	1038.857	1	0	1	8.87	1	0	1	.007	1	0	1
474			min	-145.766	3	0	1	-238.025	4	0	1	-.199	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1039.028	1	0	1	8.87	1	0	1	.008	1	0	1
476			min	-145.639	3	0	1	-238.172	4	0	1	-.226	4	0	1
477		11	max	1039.198	1	0	1	8.87	1	0	1	.009	1	0	1
478			min	-145.511	3	0	1	-238.32	4	0	1	-.253	4	0	1
479		12	max	1039.368	1	0	1	8.87	1	0	1	.01	1	0	1
480			min	-145.383	3	0	1	-238.467	4	0	1	-.281	4	0	1
481		13	max	1039.539	1	0	1	8.87	1	0	1	.011	1	0	1
482			min	-145.255	3	0	1	-238.615	4	0	1	-.308	4	0	1
483		14	max	1039.709	1	0	1	8.87	1	0	1	.012	1	0	1
484			min	-145.128	3	0	1	-238.763	4	0	1	-.336	4	0	1
485		15	max	1039.879	1	0	1	8.87	1	0	1	.013	1	0	1
486			min	-145	3	0	1	-238.91	4	0	1	-.363	4	0	1
487		16	max	1040.05	1	0	1	8.87	1	0	1	.014	1	0	1
488			min	-144.872	3	0	1	-239.058	4	0	1	-.39	4	0	1
489		17	max	1040.22	1	0	1	8.87	1	0	1	.015	1	0	1
490			min	-144.744	3	0	1	-239.206	4	0	1	-.418	4	0	1
491		18	max	1040.39	1	0	1	8.87	1	0	1	.016	1	0	1
492			min	-144.617	3	0	1	-239.353	4	0	1	-.445	4	0	1
493		19	max	1040.561	1	0	1	8.87	1	0	1	.017	1	0	1
494			min	-144.489	3	0	1	-239.501	4	0	1	-.473	4	0	1
495	M1	1	max	146.178	1	797.934	3	43.241	5	0	2	.143	1	0	15
496			min	-18.778	5	-442.933	2	-51.104	1	0	3	-.098	5	-.014	2
497		2	max	146.89	1	796.789	3	44.702	5	0	2	.111	1	.262	2
498			min	-18.446	5	-444.46	2	-51.104	1	0	3	-.071	5	-.502	3
499		3	max	567.127	3	580.994	2	14.896	5	0	3	.079	1	.527	2
500			min	-347.102	2	-613.332	3	-50.829	1	0	2	-.043	5	-.981	3
501		4	max	567.661	3	579.467	2	16.357	5	0	3	.048	1	.172	1
502			min	-346.39	2	-614.477	3	-50.829	1	0	2	-.033	5	-.6	3
503		5	max	568.195	3	577.94	2	17.817	5	0	3	.016	1	-.005	15
504			min	-345.678	2	-615.622	3	-50.829	1	0	2	-.022	5	-.218	3
505		6	max	568.729	3	576.413	2	19.277	5	0	3	-.001	10	.164	3
506			min	-344.966	2	-616.767	3	-50.829	1	0	2	-.015	1	-.551	2
507		7	max	569.263	3	574.886	2	20.737	5	0	3	.002	5	.547	3
508			min	-344.254	2	-617.912	3	-50.829	1	0	2	-.047	1	-.908	2
509		8	max	569.797	3	573.359	2	22.197	5	0	3	.015	5	.931	3
510			min	-343.542	2	-619.058	3	-50.829	1	0	2	-.079	1	-1.264	2
511		9	max	583.411	3	48.284	2	51.431	5	0	9	.052	1	1.086	3
512			min	-283.19	2	.457	15	-84.957	1	0	3	-.119	5	-1.441	2
513		10	max	583.945	3	46.757	2	52.891	5	0	9	0	10	1.062	3
514			min	-282.478	2	-.008	5	-84.957	1	0	3	-.087	4	-1.471	2
515		11	max	584.479	3	45.23	2	54.351	5	0	9	-.004	10	1.038	3
516			min	-281.766	2	-1.932	4	-84.957	1	0	3	-.065	4	-1.499	2
517		12	max	597.782	3	407.117	3	137.281	5	0	2	.077	1	.911	3
518			min	-221.28	2	-675.867	2	-49.23	1	0	3	-.223	5	-1.331	2
519		13	max	598.316	3	405.972	3	138.741	5	0	2	.047	1	.659	3
520			min	-220.568	2	-677.394	2	-49.23	1	0	3	-.137	5	-.911	2
521		14	max	598.85	3	404.826	3	140.201	5	0	2	.016	1	.407	3
522			min	-219.856	2	-678.921	2	-49.23	1	0	3	-.05	5	-.49	2
523		15	max	599.384	3	403.681	3	141.661	5	0	2	.037	5	.156	3
524			min	-219.144	2	-680.448	2	-49.23	1	0	3	-.014	1	-.088	1
525		16	max	599.918	3	402.536	3	143.121	5	0	2	.125	5	.355	2
526			min	-218.432	2	-681.975	2	-49.23	1	0	3	-.045	1	-.094	3
527		17	max	600.452	3	401.391	3	144.581	5	0	2	.215	5	.778	2
528			min	-217.72	2	-683.502	2	-49.23	1	0	3	-.076	1	-.343	3
529		18	max	31.753	5	644.288	2	-4.591	12	0	5	.197	5	.393	2
530			min	-147.511	1	-293.253	3	-93.556	4	0	2	-.109	1	-.169	3
531		19	max	32.085	5	642.761	2	-4.591	12	0	5	.148	5	.013	3





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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532			min	-146.799	1	-294.398	3	-92.096	4	0	2	-.145	1	-.008	1
533	M5	1	max	332.072	1	2623.957	3	78.148	5	0	1	0	1	.027	2
534			min	9.112	12	-1543.134	2	0	1	0	4	-.195	4	0	15
535		2	max	332.784	1	2622.812	3	79.608	5	0	1	0	1	.985	2
536			min	9.468	12	-1544.661	2	0	1	0	4	-.146	4	-1.614	3
537		3	max	1721.202	3	1513.128	2	52.194	4	0	4	0	1	1.911	2
538			min	-1081.106	2	-1758.21	3	0	1	0	1	-.097	4	-3.193	3
539		4	max	1721.736	3	1511.601	2	53.654	4	0	4	0	1	.972	2
540			min	-1080.394	2	-1759.355	3	0	1	0	1	-.064	4	-2.102	3
541		5	max	1722.27	3	1510.074	2	55.114	4	0	4	0	1	.079	1
542			min	-1079.682	2	-1760.5	3	0	1	0	1	-.03	4	-1.009	3
543		6	max	1722.804	3	1508.547	2	56.574	4	0	4	.004	4	.084	3
544			min	-1078.97	2	-1761.645	3	0	1	0	1	0	1	-.902	2
545		7	max	1723.338	3	1507.02	2	58.034	4	0	4	.04	4	1.177	3
546			min	-1078.258	2	-1762.79	3	0	1	0	1	0	1	-1.838	2
547		8	max	1723.872	3	1505.493	2	59.494	4	0	4	.077	4	2.272	3
548			min	-1077.546	2	-1763.936	3	0	1	0	1	0	1	-2.773	2
549		9	max	1736.622	3	164.451	2	172.892	4	0	1	0	1	2.623	3
550			min	-943.614	2	.46	15	0	1	0	1	-.182	4	-3.175	2
551		10	max	1737.156	3	162.924	2	174.352	4	0	1	0	1	2.527	3
552			min	-942.902	2	-.001	15	0	1	0	1	-.074	4	-3.277	2
553		11	max	1737.69	3	161.397	2	175.812	4	0	1	.034	4	2.432	3
554			min	-942.19	2	-1.771	6	0	1	0	1	0	1	-3.378	2
555		12	max	1751.062	3	1108.013	3	189.343	4	0	1	0	1	2.124	3
556			min	-808.526	2	-1833.253	2	0	1	0	4	-.314	4	-3.016	2
557		13	max	1751.596	3	1106.868	3	190.803	4	0	1	0	1	1.437	3
558			min	-807.814	2	-1834.78	2	0	1	0	4	-.196	4	-1.878	2
559		14	max	1752.13	3	1105.723	3	192.263	4	0	1	0	1	.75	3
560			min	-807.102	2	-1836.307	2	0	1	0	4	-.077	4	-.738	2
561		15	max	1752.664	3	1104.578	3	193.724	4	0	1	.042	4	.402	2
562			min	-806.39	2	-1837.834	2	0	1	0	4	0	1	0	15
563		16	max	1753.198	3	1103.433	3	195.184	4	0	1	.163	4	1.543	2
564			min	-805.678	2	-1839.361	2	0	1	0	4	0	1	-.621	3
565		17	max	1753.732	3	1102.287	3	196.644	4	0	1	.285	4	2.685	2
566			min	-804.966	2	-1840.888	2	0	1	0	4	0	1	-1.305	3
567		18	max	-10.697	12	2187.541	2	0	1	0	4	.301	4	1.371	2
568			min	-331.543	1	-1049.303	3	-22.631	5	0	1	0	1	-.678	3
569		19	max	-10.341	12	2186.014	2	0	1	0	4	.288	4	.015	1
570			min	-330.831	1	-1050.448	3	-21.171	5	0	1	0	1	-.026	3
571	M9	1	max	146.178	1	797.934	3	61.614	4	0	3	-.011	10	0	15
572			min	8.236	12	-442.933	2	3.958	10	0	4	-.15	4	-.014	2
573		2	max	146.89	1	796.789	3	63.074	4	0	3	-.009	10	.262	2
574			min	8.592	12	-444.46	2	3.958	10	0	4	-.111	4	-.502	3
575		3	max	567.127	3	580.994	2	50.829	1	0	2	-.006	10	.527	2
576			min	-347.102	2	-613.332	3	3.928	10	0	3	-.079	1	-.981	3
577		4	max	567.661	3	579.467	2	50.829	1	0	2	-.004	10	.172	1
578			min	-346.39	2	-614.477	3	3.928	10	0	3	-.051	4	-.6	3
579		5	max	568.195	3	577.94	2	50.829	1	0	2	-.001	10	-.005	15
580			min	-345.678	2	-615.622	3	3.928	10	0	3	-.028	4	-.218	3
581		6	max	568.729	3	576.413	2	50.829	1	0	2	.015	1	.164	3
582			min	-344.966	2	-616.767	3	3.928	10	0	3	-.009	5	-.551	2
583		7	max	569.263	3	574.886	2	50.829	1	0	2	.047	1	.547	3
584			min	-344.254	2	-617.912	3	3.928	10	0	3	.004	10	-.908	2
585		8	max	569.797	3	573.359	2	50.829	1	0	2	.079	1	.931	3
586			min	-343.542	2	-619.058	3	3.928	10	0	3	.006	10	-1.264	2
587		9	max	583.411	3	48.284	2	84.957	1	0	3	-.004	10	1.086	3
588			min	-283.19	2	.473	15	6.866	10	0	9	-.138	4	-1.441	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	583.945	3	46.757	2	84.957	1	0	3	0	1	1.062	3
590		min	-282.478	2	.013	15	6.866	10	0	9	-.087	4	-1.471	2
591	11	max	584.479	3	45.23	2	85.001	4	0	3	.054	1	1.038	3
592		min	-281.766	2	-1.808	6	6.866	10	0	9	-.047	5	-1.499	2
593	12	max	597.782	3	407.117	3	156.345	4	0	3	-.006	12	.911	3
594		min	-221.28	2	-675.867	2	3.925	12	0	2	-.252	4	-1.331	2
595	13	max	598.316	3	405.972	3	157.805	4	0	3	-.004	10	.659	3
596		min	-220.568	2	-677.394	2	3.925	12	0	2	-.155	4	-.911	2
597	14	max	598.85	3	404.826	3	159.265	4	0	3	-.001	10	.407	3
598		min	-219.856	2	-678.921	2	3.925	12	0	2	-.056	4	-.49	2
599	15	max	599.384	3	403.681	3	160.725	4	0	3	.043	4	.156	3
600		min	-219.144	2	-680.448	2	3.925	12	0	2	.001	12	-.088	1
601	16	max	599.918	3	402.536	3	162.186	4	0	3	.143	4	.355	2
602		min	-218.432	2	-681.975	2	3.925	12	0	2	.003	12	-.094	3
603	17	max	600.452	3	401.391	3	163.646	4	0	3	.244	4	.778	2
604		min	-217.72	2	-683.502	2	3.925	12	0	2	.006	12	-.343	3
605	18	max	-7.977	12	644.288	2	57.685	1	0	2	.238	4	.393	2
606		min	-147.511	1	-293.253	3	-73.002	5	0	3	.009	12	-.169	3
607	19	max	-7.621	12	642.761	2	57.685	1	0	2	.201	4	.013	3
608		min	-146.799	1	-294.398	3	-71.542	5	0	3	.011	12	-.008	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.226	2	.011	3	1.548e-2	2	NC	1	NC	1
2				min	-619	4	-.062	3	-.007	2	-4.129e-3	3	NC	1	NC
3		2	max	0	1	.165	2	.014	1	1.645e-2	2	NC	4	NC	1
4			min	-619	4	.005	15	-.01	5	-3.616e-3	3	1134.18	3	NC	1
5		3	max	0	1	.217	3	.033	1	1.741e-2	2	NC	5	NC	2
6			min	-619	4	.003	15	-.014	5	-3.103e-3	3	622.763	3	5154.845	1
7		4	max	0	1	.297	3	.048	1	1.838e-2	2	NC	5	NC	2
8			min	-619	4	.003	15	-.011	5	-2.59e-3	3	484.086	3	3538.189	1
9		5	max	0	1	.322	3	.055	1	1.934e-2	2	NC	5	NC	2
10			min	-619	4	.003	15	-.004	5	-2.078e-3	3	452.246	3	3103.773	1
11		6	max	0	1	.294	3	.051	1	2.031e-2	2	NC	4	NC	2
12			min	-619	4	.003	15	-.004	10	-1.565e-3	3	488.416	3	3324.386	1
13		7	max	0	1	.222	3	.038	1	2.127e-2	2	NC	4	NC	2
14			min	-619	4	.005	15	-.007	10	-1.052e-3	3	611.337	3	4479.369	1
15		8	max	0	1	.272	2	.032	3	2.224e-2	2	NC	4	NC	2
16			min	-619	4	.006	15	-.01	10	-5.395e-4	3	914.646	3	8464.838	3
17		9	max	0	1	.331	2	.032	3	2.32e-2	2	NC	4	NC	1
18			min	-619	4	.007	15	-.019	2	-2.674e-5	3	1665.734	2	8208.181	3
19		10	max	0	1	.357	2	.032	3	2.417e-2	2	NC	4	NC	1
20			min	-619	4	.001	3	-.023	2	4.86e-4	3	1333.91	2	8159.952	3
21		11	max	0	10	.331	2	.032	3	2.32e-2	2	NC	4	NC	1
22			min	-619	4	.007	15	-.019	2	-2.674e-5	3	1665.734	2	8208.181	3
23		12	max	0	10	.272	2	.032	3	2.224e-2	2	NC	4	NC	2
24			min	-619	4	.006	15	-.01	10	-5.395e-4	3	914.646	3	8464.838	3
25		13	max	0	10	.222	3	.038	1	2.127e-2	2	NC	4	NC	2
26			min	-619	4	.004	15	-.007	10	-1.052e-3	3	611.337	3	4479.369	1
27		14	max	0	10	.294	3	.051	1	2.031e-2	2	NC	4	NC	2
28			min	-619	4	.003	15	-.004	10	-1.565e-3	3	488.416	3	3324.386	1
29		15	max	0	10	.322	3	.055	1	1.934e-2	2	NC	5	NC	2
30			min	-619	4	.002	15	-.002	10	-2.078e-3	3	452.246	3	3103.773	1
31		16	max	0	10	.297	3	.048	1	1.838e-2	2	NC	5	NC	2
32			min	-619	4	.002	15	-.002	10	-2.59e-3	3	484.086	3	3538.189	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	10	.217	3	.033	1	1.741e-2	2	NC	5	NC	2
34		min	-.619	4	.002	15	-.002	10	-3.103e-3	3	622.763	3	5154.845	1
35	18	max	0	10	.165	2	.017	4	1.645e-2	2	NC	4	NC	1
36		min	-.619	4	.003	15	-.004	10	-3.616e-3	3	1134.18	3	9692.503	4
37	19	max	0	10	.226	2	.011	3	1.548e-2	2	NC	1	NC	1
38		min	-.619	4	-.062	3	-.007	2	-4.129e-3	3	NC	1	NC	1
39	M14	1	max	0	.454	3	.01	3	8.564e-3	2	NC	1	NC	1
40		min	-.468	4	-.669	2	-.006	2	-6.84e-3	3	NC	1	NC	1
41	2	max	0	1	.655	3	.011	3	9.729e-3	2	NC	5	NC	1
42		min	-.468	4	-.873	2	-.016	5	-7.897e-3	3	853.067	2	NC	1
43	3	max	0	1	.833	3	.024	1	1.089e-2	2	NC	5	NC	2
44		min	-.468	4	-1.058	2	-.02	5	-8.955e-3	3	447.006	2	6943.082	1
45	4	max	0	1	.972	3	.039	1	1.206e-2	2	NC	5	NC	2
46		min	-.468	4	-1.211	2	-.015	5	-1.001e-2	3	320.844	2	4380.731	1
47	5	max	0	1	1.063	3	.047	1	1.322e-2	2	NC	15	NC	2
48		min	-.468	4	-1.324	2	-.004	5	-1.107e-2	3	265.557	2	3664.373	1
49	6	max	0	1	1.106	3	.045	1	1.439e-2	2	NC	15	NC	2
50		min	-.468	4	-1.394	2	-.004	10	-1.213e-2	3	239.781	2	3804.48	1
51	7	max	0	1	1.105	3	.034	1	1.555e-2	2	NC	15	NC	2
52		min	-.468	4	-1.426	2	-.006	10	-1.318e-2	3	229.881	2	5007.927	1
53	8	max	0	1	1.074	3	.03	4	1.672e-2	2	NC	15	NC	2
54		min	-.468	4	-1.427	2	-.009	10	-1.424e-2	3	229.587	2	5654.989	4
55	9	max	0	1	1.035	3	.028	3	1.788e-2	2	NC	15	NC	1
56		min	-.468	4	-1.412	2	-.017	2	-1.53e-2	3	234.16	2	7921.621	4
57	10	max	0	1	1.014	3	.029	3	1.905e-2	2	NC	15	NC	1
58		min	-.468	4	-1.401	2	-.02	2	-1.636e-2	3	237.499	2	9218.866	3
59	11	max	0	12	1.035	3	.028	3	1.788e-2	2	NC	15	NC	1
60		min	-.468	4	-1.412	2	-.017	2	-1.53e-2	3	234.16	2	9294.203	3
61	12	max	0	12	1.074	3	.028	3	1.672e-2	2	NC	15	NC	2
62		min	-.468	4	-1.427	2	-.02	5	-1.424e-2	3	229.587	2	9652.669	3
63	13	max	0	12	1.105	3	.034	1	1.555e-2	2	NC	15	NC	2
64		min	-.468	4	-1.426	2	-.014	5	-1.318e-2	3	229.881	2	5007.927	1
65	14	max	0	12	1.106	3	.045	1	1.439e-2	2	NC	15	NC	2
66		min	-.468	4	-1.394	2	-.004	10	-1.213e-2	3	239.781	2	3804.48	1
67	15	max	0	12	1.063	3	.047	1	1.322e-2	2	NC	15	NC	2
68		min	-.468	4	-1.324	2	-.002	10	-1.107e-2	3	265.557	2	3664.373	1
69	16	max	0	12	.972	3	.039	1	1.206e-2	2	NC	5	NC	2
70		min	-.468	4	-1.211	2	-.002	10	-1.001e-2	3	320.844	2	4380.731	1
71	17	max	0	12	.833	3	.031	4	1.089e-2	2	NC	5	NC	2
72		min	-.468	4	-1.058	2	-.002	10	-8.955e-3	3	447.006	2	5421.66	4
73	18	max	0	12	.655	3	.021	4	9.729e-3	2	NC	5	NC	1
74		min	-.468	4	-.873	2	-.003	10	-7.897e-3	3	853.067	2	7915.415	4
75	19	max	0	12	.454	3	.01	3	8.564e-3	2	NC	1	NC	1
76		min	-.468	4	-.669	2	-.006	2	-6.84e-3	3	NC	1	NC	1
77	M15	1	max	0	.465	3	.009	3	5.76e-3	3	NC	1	NC	1
78		min	-.384	4	-.667	2	-.006	2	-8.883e-3	2	NC	1	NC	1
79	2	max	0	10	.616	3	.01	3	6.635e-3	3	NC	5	NC	1
80		min	-.384	4	-.906	2	-.024	5	-1.01e-2	2	728.068	2	6764.502	5
81	3	max	0	10	.753	3	.025	1	7.51e-3	3	NC	5	NC	2
82		min	-.384	4	-1.12	2	-.03	5	-1.132e-2	2	384.407	2	5436.73	5
83	4	max	0	10	.869	3	.039	1	8.385e-3	3	NC	5	NC	2
84		min	-.384	4	-1.29	2	-.023	5	-1.253e-2	2	279.312	2	4352.003	1
85	5	max	0	10	.956	3	.047	1	9.26e-3	3	NC	15	NC	2
86		min	-.384	4	-1.407	2	-.008	5	-1.375e-2	2	235.162	2	3639.01	1
87	6	max	0	10	1.014	3	.046	1	1.013e-2	3	NC	15	NC	2
88		min	-.384	4	-1.469	2	-.003	10	-1.496e-2	2	217.097	2	3772.39	1
89	7	max	0	10	1.043	3	.035	1	1.101e-2	3	NC	15	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-0.384	4	-1.481	2	-0.006	10	-1.618e-2	2	213.857	2	4945.913	1
91	8	max	0	10	1.05	3	.037	4	1.188e-2	3	NC	15	NC	2
92		min	-0.384	4	-1.458	2	-.009	10	-1.74e-2	2	220.171	2	4581.189	4
93	9	max	0	10	1.044	3	.027	4	1.276e-2	3	NC	15	NC	1
94		min	-0.384	4	-1.421	2	-.016	2	-1.861e-2	2	230.944	2	6212.263	4
95	10	max	0	1	1.039	3	.026	3	1.363e-2	3	NC	15	NC	1
96		min	-0.384	4	-1.4	2	-.019	2	-1.983e-2	2	237.393	2	9982.767	3
97	11	max	0	1	1.044	3	.026	3	1.276e-2	3	NC	15	NC	1
98		min	-0.384	4	-1.421	2	-.023	5	-1.861e-2	2	230.944	2	7621.104	5
99	12	max	0	1	1.05	3	.026	3	1.188e-2	3	NC	15	NC	2
100		min	-0.384	4	-1.458	2	-.027	5	-1.74e-2	2	220.171	2	6472.899	5
101	13	max	0	1	1.043	3	.035	1	1.101e-2	3	NC	15	NC	2
102		min	-0.384	4	-1.481	2	-.018	5	-1.618e-2	2	213.857	2	4945.913	1
103	14	max	0	1	1.014	3	.046	1	1.013e-2	3	NC	15	NC	2
104		min	-0.384	4	-1.469	2	-.003	10	-1.496e-2	2	217.097	2	3772.39	1
105	15	max	0	1	.956	3	.047	1	9.26e-3	3	NC	15	NC	2
106		min	-0.384	4	-1.407	2	-.002	10	-1.375e-2	2	235.162	2	3639.01	1
107	16	max	0	1	.869	3	.039	1	8.385e-3	3	NC	5	NC	2
108		min	-0.384	4	-1.29	2	-.002	10	-1.253e-2	2	279.312	2	4342.917	4
109	17	max	0	1	.753	3	.041	4	7.51e-3	3	NC	5	NC	2
110		min	-0.384	4	-1.12	2	-.002	10	-1.132e-2	2	384.407	2	4154.517	4
111	18	max	0	1	.616	3	.028	4	6.635e-3	3	NC	5	NC	1
112		min	-0.384	4	-.906	2	-.003	10	-1.01e-2	2	728.068	2	5892.417	4
113	19	max	0	1	.465	3	.009	3	5.76e-3	3	NC	1	NC	1
114		min	-0.384	4	-.667	2	-.006	2	-8.883e-3	2	NC	1	NC	1
115	M16	1	max	0	.203	2	.008	3	1.118e-2	3	NC	1	NC	1
116		min	-.124	4	-.166	3	-.005	2	-1.323e-2	2	NC	1	NC	1
117	2	max	0	12	.106	1	.014	1	1.211e-2	3	NC	4	NC	1
118		min	-.124	4	-.133	3	-.017	5	-1.372e-2	2	1686.847	2	9513.884	5
119	3	max	0	12	.044	1	.033	1	1.303e-2	3	NC	5	NC	2
120		min	-.124	4	-.109	3	-.022	5	-1.42e-2	2	943.942	2	5154.183	1
121	4	max	0	12	.021	9	.049	1	1.396e-2	3	NC	5	NC	2
122		min	-.124	4	-.102	3	-.018	5	-1.469e-2	2	760.149	2	3519.075	1
123	5	max	0	12	.023	9	.056	1	1.489e-2	3	NC	5	NC	3
124		min	-.124	4	-.115	3	-.009	5	-1.517e-2	2	755.853	2	3069.084	1
125	6	max	0	12	.049	1	.053	1	1.582e-2	3	NC	4	NC	2
126		min	-.124	4	-.146	3	-.002	10	-1.565e-2	2	917.858	2	3258.92	1
127	7	max	0	12	.11	1	.04	1	1.675e-2	3	NC	4	NC	2
128		min	-.124	4	-.19	3	-.004	10	-1.614e-2	2	1495.71	2	4316.25	1
129	8	max	0	12	.183	1	.023	14	1.768e-2	3	NC	1	NC	2
130		min	-.124	4	-.239	3	-.007	10	-1.662e-2	2	2390.105	3	7140.753	4
131	9	max	0	12	.254	2	.023	3	1.861e-2	3	NC	4	NC	1
132		min	-.124	4	-.281	3	-.014	2	-1.711e-2	2	1518.265	3	NC	1
133	10	max	0	1	.289	2	.023	3	1.954e-2	3	NC	4	NC	1
134		min	-.124	4	-.299	3	-.017	2	-1.759e-2	2	1309.189	3	NC	1
135	11	max	0	1	.254	2	.023	3	1.861e-2	3	NC	4	NC	1
136		min	-.124	4	-.281	3	-.014	2	-1.711e-2	2	1518.265	3	NC	1
137	12	max	0	1	.183	1	.023	3	1.768e-2	3	NC	1	NC	2
138		min	-.124	4	-.239	3	-.013	5	-1.662e-2	2	2390.105	3	8237.082	1
139	13	max	0	1	.11	1	.04	1	1.675e-2	3	NC	4	NC	2
140		min	-.124	4	-.19	3	-.006	5	-1.614e-2	2	1495.71	2	4316.25	1
141	14	max	0	1	.049	1	.053	1	1.582e-2	3	NC	4	NC	2
142		min	-.124	4	-.146	3	-.002	10	-1.565e-2	2	917.858	2	3258.92	1
143	15	max	0	1	.023	9	.056	1	1.489e-2	3	NC	5	NC	3
144		min	-.124	4	-.115	3	0	10	-1.517e-2	2	755.853	2	3069.084	1
145	16	max	0	1	.021	9	.049	1	1.396e-2	3	NC	5	NC	2
146		min	-.124	4	-.102	3	0	10	-1.469e-2	2	760.149	2	3519.075	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.044	1	.035	4	1.303e-2	3	NC	5	NC	2
148			min	-.124	4	-.109	3	0	10	-1.42e-2	2	943.942	2	4822.591	4
149		18	max	0	1	.106	1	.023	4	1.211e-2	3	NC	4	NC	1
150			min	-.124	4	-.133	3	-.002	10	-1.372e-2	2	1686.847	2	7284.985	4
151		19	max	0	1	.203	2	.008	3	1.118e-2	3	NC	1	NC	1
152			min	-.124	4	-.166	3	-.005	2	-1.323e-2	2	NC	1	NC	1
153	M2	1	max	.007	2	.01	2	.007	1	2.091e-3	5	NC	1	NC	1
154			min	-.01	3	-.016	3	-.582	4	-1.455e-4	1	6802.241	2	118.655	4
155		2	max	.007	2	.009	2	.006	1	2.107e-3	5	NC	1	NC	1
156			min	-.009	3	-.015	3	-.535	4	-1.374e-4	1	7830.75	2	129.216	4
157		3	max	.007	2	.008	2	.006	1	2.122e-3	5	NC	1	NC	1
158			min	-.009	3	-.015	3	-.487	4	-1.294e-4	1	9206.942	2	141.761	4
159		4	max	.006	2	.006	2	.005	1	2.138e-3	5	NC	1	NC	1
160			min	-.008	3	-.014	3	-.441	4	-1.213e-4	1	NC	1	156.811	4
161		5	max	.006	2	.005	2	.004	1	2.154e-3	5	NC	1	NC	1
162			min	-.008	3	-.013	3	-.395	4	-1.132e-4	1	NC	1	175.073	4
163		6	max	.005	2	.004	2	.004	1	2.169e-3	5	NC	1	NC	1
164			min	-.007	3	-.013	3	-.35	4	-1.052e-4	1	NC	1	197.526	4
165		7	max	.005	2	.003	2	.003	1	2.185e-3	5	NC	1	NC	1
166			min	-.007	3	-.012	3	-.306	4	-9.71e-5	1	NC	1	225.565	4
167		8	max	.005	2	.002	2	.003	1	2.202e-3	4	NC	1	NC	1
168			min	-.006	3	-.012	3	-.265	4	-8.903e-5	1	NC	1	261.223	4
169		9	max	.004	2	0	2	.003	1	2.22e-3	4	NC	1	NC	1
170			min	-.006	3	-.011	3	-.225	4	-8.096e-5	1	NC	1	307.568	4
171		10	max	.004	2	0	2	.002	1	2.238e-3	4	NC	1	NC	1
172			min	-.005	3	-.01	3	-.187	4	-7.29e-5	1	NC	1	369.406	4
173		11	max	.003	2	0	2	.002	1	2.255e-3	4	NC	1	NC	1
174			min	-.004	3	-.009	3	-.152	4	-6.483e-5	1	NC	1	454.615	4
175		12	max	.003	2	0	15	.001	1	2.273e-3	4	NC	1	NC	1
176			min	-.004	3	-.008	3	-.12	4	-5.676e-5	1	NC	1	576.868	4
177		13	max	.002	2	0	15	0	1	2.291e-3	4	NC	1	NC	1
178			min	-.003	3	-.007	3	-.091	4	-4.869e-5	1	NC	1	761.645	4
179		14	max	.002	2	0	15	0	1	2.309e-3	4	NC	1	NC	1
180			min	-.003	3	-.006	3	-.065	4	-4.063e-5	1	NC	1	1061.055	4
181		15	max	.002	2	0	15	0	1	2.327e-3	4	NC	1	NC	1
182			min	-.002	3	-.005	3	-.043	4	-3.256e-5	1	NC	1	1596.106	4
183		16	max	.001	2	0	15	0	1	2.345e-3	4	NC	1	NC	1
184			min	-.002	3	-.004	3	-.026	4	-2.449e-5	1	NC	1	2704.949	4
185		17	max	0	2	0	15	0	1	2.363e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	3	-.012	4	-1.642e-5	1	NC	1	5663.907	4
187		18	max	0	2	0	15	0	1	2.381e-3	4	NC	1	NC	1
188			min	0	3	-.001	3	-.004	4	-8.355e-6	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.399e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-8.17e-7	3	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	5.517e-8	3	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.979e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	9.023e-5	4	NC	1	NC	1
194			min	0	2	-.003	6	0	3	1.385e-6	12	NC	1	NC	1
195		3	max	0	3	-.001	15	.025	4	6.784e-4	4	NC	1	NC	1
196			min	0	2	-.005	6	0	3	2.737e-6	12	NC	1	NC	1
197		4	max	.001	3	-.002	15	.037	4	1.267e-3	4	NC	1	NC	1
198			min	-.001	2	-.008	6	0	3	4.089e-6	12	NC	1	NC	1
199		5	max	.002	3	-.002	15	.048	4	1.855e-3	4	NC	1	NC	1
200			min	-.002	2	-.011	6	0	3	5.442e-6	12	8992.505	6	NC	1
201		6	max	.002	3	-.003	15	.058	4	2.443e-3	4	NC	2	NC	1
202			min	-.002	2	-.014	6	0	12	6.794e-6	12	7244.349	6	NC	1
203		7	max	.003	3	-.004	15	.067	4	3.031e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.016	6	0	12	8.146e-6	12	6193.747	6	NC	1
205		8	max	.003	3	-.004	15	.076	4	3.619e-3	4	NC	5	NC	1
206			min	-.003	2	-.018	6	0	12	9.499e-6	12	5545.308	6	NC	1
207		9	max	.004	3	-.004	15	.085	4	4.207e-3	4	NC	5	NC	1
208			min	-.003	2	-.02	6	0	12	1.085e-5	12	5160.165	6	NC	1
209		10	max	.004	3	-.005	15	.093	4	4.795e-3	4	NC	5	NC	1
210			min	-.004	2	-.021	6	0	12	1.22e-5	12	4970.746	6	NC	1
211		11	max	.005	3	-.005	15	.101	4	5.384e-3	4	NC	5	NC	1
212			min	-.004	2	-.021	6	0	12	1.356e-5	12	4948.976	6	NC	1
213		12	max	.005	3	-.004	15	.109	4	5.972e-3	4	NC	5	NC	1
214			min	-.004	2	-.02	6	0	12	1.491e-5	12	5095.491	6	NC	1
215		13	max	.006	3	-.004	15	.117	4	6.56e-3	4	NC	5	NC	1
216			min	-.005	2	-.019	6	0	12	1.626e-5	12	5441.243	6	NC	1
217		14	max	.006	3	-.004	15	.126	4	7.148e-3	4	NC	5	NC	1
218			min	-.005	2	-.017	6	0	12	1.761e-5	12	6063.716	6	NC	1
219		15	max	.007	3	-.003	15	.134	4	7.736e-3	4	NC	3	NC	1
220			min	-.006	2	-.014	6	0	12	1.896e-5	12	7135.318	6	NC	1
221		16	max	.007	3	-.002	15	.144	4	8.324e-3	4	NC	1	NC	1
222			min	-.006	2	-.011	6	0	12	2.032e-5	12	9074.122	6	NC	1
223		17	max	.008	3	-.001	15	.154	4	8.912e-3	4	NC	1	NC	1
224			min	-.006	2	-.008	6	0	12	2.167e-5	12	NC	1	NC	1
225		18	max	.008	3	0	15	.166	4	9.501e-3	4	NC	1	NC	1
226			min	-.007	2	-.005	1	0	12	2.302e-5	12	NC	1	NC	1
227		19	max	.009	3	0	5	.179	4	1.009e-2	4	NC	1	NC	1
228			min	-.007	2	-.002	1	0	12	2.437e-5	12	NC	1	NC	1
229	M4	1	max	.002	1	.007	2	0	12	8.768e-5	1	NC	1	NC	2
230			min	0	3	-.009	3	-.179	4	-1.415e-4	5	NC	1	138.909	4
231		2	max	.002	1	.006	2	0	12	8.768e-5	1	NC	1	NC	2
232			min	0	3	-.008	3	-.164	4	-1.415e-4	5	NC	1	151.089	4
233		3	max	.002	1	.006	2	0	12	8.768e-5	1	NC	1	NC	2
234			min	0	3	-.008	3	-.15	4	-1.415e-4	5	NC	1	165.582	4
235		4	max	.002	1	.006	2	0	12	8.768e-5	1	NC	1	NC	2
236			min	0	3	-.007	3	-.136	4	-1.415e-4	5	NC	1	182.989	4
237		5	max	.002	1	.005	2	0	12	8.768e-5	1	NC	1	NC	2
238			min	0	3	-.007	3	-.122	4	-1.415e-4	5	NC	1	204.127	4
239		6	max	.002	1	.005	2	0	12	8.768e-5	1	NC	1	NC	2
240			min	0	3	-.006	3	-.108	4	-1.415e-4	5	NC	1	230.128	4
241		7	max	.002	1	.005	2	0	12	8.768e-5	1	NC	1	NC	2
242			min	0	3	-.006	3	-.094	4	-1.415e-4	5	NC	1	262.601	4
243		8	max	.002	1	.004	2	0	12	8.768e-5	1	NC	1	NC	2
244			min	0	3	-.005	3	-.082	4	-1.415e-4	5	NC	1	303.892	4
245		9	max	.001	1	.004	2	0	12	8.768e-5	1	NC	1	NC	1
246			min	0	3	-.005	3	-.069	4	-1.415e-4	5	NC	1	357.531	4
247		10	max	.001	1	.003	2	0	12	8.768e-5	1	NC	1	NC	1
248			min	0	3	-.004	3	-.058	4	-1.415e-4	5	NC	1	429.038	4
249		11	max	.001	1	.003	2	0	12	8.768e-5	1	NC	1	NC	1
250			min	0	3	-.004	3	-.047	4	-1.415e-4	5	NC	1	527.441	4
251		12	max	0	1	.003	2	0	12	8.768e-5	1	NC	1	NC	1
252			min	0	3	-.003	3	-.037	4	-1.415e-4	5	NC	1	668.361	4
253		13	max	0	1	.002	2	0	12	8.768e-5	1	NC	1	NC	1
254			min	0	3	-.003	3	-.028	4	-1.415e-4	5	NC	1	880.787	4
255		14	max	0	1	.002	2	0	12	8.768e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	-.02	4	-1.415e-4	5	NC	1	1223.696	4
257		15	max	0	1	.002	2	0	12	8.768e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	-.014	4	-1.415e-4	5	NC	1	1833.036	4
259		16	max	0	1	.001	2	0	12	8.768e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	-.008	4	-1.415e-4	5	NC	1	3084.607	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	8.768e-5	1	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-1.415e-4	5	NC	1	6371.708	4
263		18	max	0	1	0	2	0	12	8.768e-5	1	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-1.415e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	8.768e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.415e-4	5	NC	1	NC	1
267	M6	1	max	.022	2	.033	2	0	1	2.177e-3	4	NC	4	NC	1
268			min	-.031	3	-.047	3	-.588	4	0	1	1472.508	3	117.491	4
269		2	max	.021	2	.03	2	0	1	2.191e-3	4	NC	4	NC	1
270			min	-.029	3	-.044	3	-.54	4	0	1	1559.04	3	127.949	4
271		3	max	.02	2	.027	2	0	1	2.204e-3	4	NC	4	NC	1
272			min	-.028	3	-.042	3	-.492	4	0	1	1656.406	3	140.373	4
273		4	max	.018	2	.025	2	0	1	2.218e-3	4	NC	4	NC	1
274			min	-.026	3	-.039	3	-.445	4	0	1	1766.805	3	155.277	4
275		5	max	.017	2	.022	2	0	1	2.232e-3	4	NC	4	NC	1
276			min	-.024	3	-.037	3	-.399	4	0	1	1893.063	3	173.361	4
277		6	max	.016	2	.019	2	0	1	2.246e-3	4	NC	4	NC	1
278			min	-.022	3	-.034	3	-.353	4	0	1	2038.873	3	195.597	4
279		7	max	.015	2	.017	2	0	1	2.26e-3	4	NC	4	NC	1
280			min	-.021	3	-.031	3	-.309	4	0	1	2209.164	3	223.364	4
281		8	max	.014	2	.014	2	0	1	2.274e-3	4	NC	1	NC	1
282			min	-.019	3	-.029	3	-.267	4	0	1	2410.653	3	258.677	4
283		9	max	.012	2	.012	2	0	1	2.287e-3	4	NC	1	NC	1
284			min	-.017	3	-.026	3	-.227	4	0	1	2652.746	3	304.574	4
285		10	max	.011	2	.01	2	0	1	2.301e-3	4	NC	1	NC	1
286			min	-.015	3	-.023	3	-.189	4	0	1	2949.031	3	365.814	4
287		11	max	.01	2	.008	2	0	1	2.315e-3	4	NC	1	NC	1
288			min	-.014	3	-.021	3	-.154	4	0	1	3319.888	3	450.199	4
289		12	max	.009	2	.006	2	0	1	2.329e-3	4	NC	1	NC	1
290			min	-.012	3	-.018	3	-.121	4	0	1	3797.344	3	571.271	4
291		13	max	.007	2	.004	2	0	1	2.343e-3	4	NC	1	NC	1
292			min	-.01	3	-.016	3	-.092	4	0	1	4434.782	3	754.262	4
293		14	max	.006	2	.003	2	0	1	2.356e-3	4	NC	1	NC	1
294			min	-.009	3	-.013	3	-.066	4	0	1	5328.289	3	1050.776	4
295		15	max	.005	2	.002	2	0	1	2.37e-3	4	NC	1	NC	1
296			min	-.007	3	-.01	3	-.044	4	0	1	6670.043	3	1580.648	4
297		16	max	.004	2	0	2	0	1	2.384e-3	4	NC	1	NC	1
298			min	-.005	3	-.008	3	-.026	4	0	1	8908.464	3	2678.737	4
299		17	max	.002	2	0	2	0	1	2.398e-3	4	NC	1	NC	1
300			min	-.003	3	-.005	3	-.012	4	0	1	NC	1	5608.883	4
301		18	max	.001	2	0	2	0	1	2.412e-3	4	NC	1	NC	1
302			min	-.002	3	-.003	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.426e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.037e-4	4	NC	1	NC	1
307		2	max	.001	3	0	2	.013	4	6.791e-5	4	NC	1	NC	1
308			min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309		3	max	.003	3	-.001	15	.026	4	6.395e-4	4	NC	1	NC	1
310			min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
311		4	max	.004	3	-.002	15	.037	4	1.211e-3	4	NC	1	NC	1
312			min	-.004	2	-.011	3	0	1	0	1	NC	1	NC	1
313		5	max	.006	3	-.003	15	.048	4	1.783e-3	4	NC	1	NC	1
314			min	-.005	2	-.014	3	0	1	0	1	8063.982	3	NC	1
315		6	max	.007	3	-.003	15	.058	4	2.354e-3	4	NC	1	NC	1
316			min	-.007	2	-.016	3	0	1	0	1	6801.084	3	9848.327	4
317		7	max	.008	3	-.004	15	.068	4	2.926e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.008	2	-.019	3	0	1	0	1	6042.488	3	9870.613	5
319	8	max	.01	3	-.004	15	.077	4	3.497e-3	4	NC	2	NC	1
320		min	-.009	2	-.02	3	0	1	0	1	5530.893	4	NC	1
321	9	max	.011	3	-.005	15	.085	4	4.069e-3	4	NC	2	NC	1
322		min	-.01	2	-.021	3	0	1	0	1	5147.641	4	NC	1
323	10	max	.013	3	-.005	15	.093	4	4.64e-3	4	NC	5	NC	1
324		min	-.012	2	-.022	3	0	1	0	1	4959.398	4	NC	1
325	11	max	.014	3	-.005	15	.101	4	5.212e-3	4	NC	5	NC	1
326		min	-.013	2	-.022	3	0	1	0	1	4938.276	4	NC	1
327	12	max	.015	3	-.005	15	.109	4	5.784e-3	4	NC	5	NC	1
328		min	-.014	2	-.021	3	0	1	0	1	5084.993	4	NC	1
329	13	max	.017	3	-.005	15	.116	4	6.355e-3	4	NC	5	NC	1
330		min	-.016	2	-.02	3	0	1	0	1	5430.496	4	NC	1
331	14	max	.018	3	-.004	15	.124	4	6.927e-3	4	NC	2	NC	1
332		min	-.017	2	-.018	3	0	1	0	1	6052.168	4	NC	1
333	15	max	.02	3	-.004	15	.132	4	7.498e-3	4	NC	1	NC	1
334		min	-.018	2	-.016	3	0	1	0	1	7122.14	4	NC	1
335	16	max	.021	3	-.003	15	.141	4	8.07e-3	4	NC	1	NC	1
336		min	-.02	2	-.014	3	0	1	0	1	9057.773	4	NC	1
337	17	max	.022	3	-.002	15	.15	4	8.641e-3	4	NC	1	NC	1
338		min	-.021	2	-.011	3	0	1	0	1	NC	1	NC	1
339	18	max	.024	3	-.001	15	.161	4	9.213e-3	4	NC	1	NC	1
340		min	-.022	2	-.008	3	0	1	0	1	NC	1	NC	1
341	19	max	.025	3	0	10	.172	4	9.785e-3	4	NC	1	NC	1
342		min	-.023	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	2	.023	2	0	0	1	NC	1	NC	1
344		min	-.001	3	-.026	3	-.172	4	-2.656e-4	4	NC	1	143.945	4
345	2	max	.006	2	.021	2	0	1	0	1	NC	1	NC	1
346		min	-.001	3	-.024	3	-.158	4	-2.656e-4	4	NC	1	156.582	4
347	3	max	.006	2	.02	2	0	1	0	1	NC	1	NC	1
348		min	-.001	3	-.023	3	-.145	4	-2.656e-4	4	NC	1	171.618	4
349	4	max	.006	2	.019	2	0	1	0	1	NC	1	NC	1
350		min	-.001	3	-.021	3	-.131	4	-2.656e-4	4	NC	1	189.676	4
351	5	max	.005	2	.018	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.02	3	-.117	4	-2.656e-4	4	NC	1	211.604	4
353	6	max	.005	2	.016	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.019	3	-.104	4	-2.656e-4	4	NC	1	238.576	4
355	7	max	.004	2	.015	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.017	3	-.091	4	-2.656e-4	4	NC	1	272.262	4
357	8	max	.004	2	.014	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.016	3	-.079	4	-2.656e-4	4	NC	1	315.094	4
359	9	max	.004	2	.013	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.014	3	-.067	4	-2.656e-4	4	NC	1	370.735	4
361	10	max	.003	2	.011	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.013	3	-.056	4	-2.656e-4	4	NC	1	444.912	4
363	11	max	.003	2	.01	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.011	3	-.045	4	-2.656e-4	4	NC	1	546.991	4
365	12	max	.003	2	.009	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.01	3	-.036	4	-2.656e-4	4	NC	1	693.176	4
367	13	max	.002	2	.008	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.009	3	-.027	4	-2.656e-4	4	NC	1	913.542	4
369	14	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.007	3	-.02	4	-2.656e-4	4	NC	1	1269.277	4
371	15	max	.001	2	.005	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.006	3	-.013	4	-2.656e-4	4	NC	1	1901.426	4
373	16	max	.001	2	.004	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.004	3	-.008	4	-2.656e-4	4	NC	1	3199.89	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	-.004	4	-2.656e-4	4	NC	1	6610.317	4
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-2.656e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-2.656e-4	4	NC	1	NC	1
381	M10	1	max	.007	2	.01	2	0	10	2.161e-3	4	NC	1	NC	1
382			min	-.01	3	-.016	3	-.586	4	1.051e-5	10	6802.241	2	117.879	4
383		2	max	.007	2	.009	2	0	10	2.174e-3	4	NC	1	NC	1
384			min	-.009	3	-.015	3	-.538	4	9.916e-6	10	7830.75	2	128.372	4
385		3	max	.007	2	.008	2	0	10	2.187e-3	4	NC	1	NC	1
386			min	-.009	3	-.015	3	-.491	4	9.322e-6	10	9206.942	2	140.836	4
387		4	max	.006	2	.006	2	0	10	2.2e-3	4	NC	1	NC	1
388			min	-.008	3	-.014	3	-.444	4	8.727e-6	10	NC	1	155.79	4
389		5	max	.006	2	.005	2	0	10	2.214e-3	4	NC	1	NC	1
390			min	-.008	3	-.013	3	-.397	4	8.133e-6	10	NC	1	173.935	4
391		6	max	.005	2	.004	2	0	10	2.227e-3	4	NC	1	NC	1
392			min	-.007	3	-.013	3	-.352	4	7.538e-6	10	NC	1	196.245	4
393		7	max	.005	2	.003	2	0	10	2.24e-3	4	NC	1	NC	1
394			min	-.007	3	-.012	3	-.308	4	6.944e-6	10	NC	1	224.105	4
395		8	max	.005	2	.002	2	0	10	2.253e-3	4	NC	1	NC	1
396			min	-.006	3	-.012	3	-.266	4	6.349e-6	10	NC	1	259.537	4
397		9	max	.004	2	0	2	0	10	2.266e-3	4	NC	1	NC	1
398			min	-.006	3	-.011	3	-.226	4	5.755e-6	10	NC	1	305.59	4
399		10	max	.004	2	0	2	0	10	2.28e-3	4	NC	1	NC	1
400			min	-.005	3	-.01	3	-.188	4	5.16e-6	10	NC	1	367.038	4
401		11	max	.003	2	0	2	0	10	2.293e-3	4	NC	1	NC	1
402			min	-.004	3	-.009	3	-.153	4	4.566e-6	10	NC	1	451.712	4
403		12	max	.003	2	-.001	2	0	10	2.306e-3	4	NC	1	NC	1
404			min	-.004	3	-.008	3	-.121	4	3.972e-6	10	NC	1	573.203	4
405		13	max	.002	2	-.002	2	0	10	2.319e-3	4	NC	1	NC	1
406			min	-.003	3	-.007	3	-.091	4	3.377e-6	10	NC	1	756.836	4
407		14	max	.002	2	-.002	15	0	10	2.332e-3	4	NC	1	NC	1
408			min	-.003	3	-.006	3	-.066	4	2.783e-6	10	NC	1	1054.408	4
409		15	max	.002	2	-.001	15	0	10	2.346e-3	4	NC	1	NC	1
410			min	-.002	3	-.005	3	-.044	4	2.188e-6	10	NC	1	1586.215	4
411		16	max	.001	2	-.001	15	0	10	2.359e-3	4	NC	1	NC	1
412			min	-.002	3	-.004	4	-.026	4	1.594e-6	10	NC	1	2688.464	4
413		17	max	0	2	0	15	0	10	2.372e-3	4	NC	1	NC	1
414			min	-.001	3	-.003	4	-.012	4	9.993e-7	10	NC	1	5630.413	4
415		18	max	0	2	0	15	0	10	2.385e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.004	4	4.049e-7	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.399e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.08e-7	2	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	9.09e-7	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.973e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	8.163e-5	5	NC	1	NC	1
422			min	0	2	-.003	4	0	2	-1.722e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	.025	4	6.58e-4	4	NC	1	NC	1
424			min	0	2	-.006	4	0	1	-3.534e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	.037	4	1.236e-3	4	NC	1	NC	1
426			min	-.001	2	-.009	4	0	1	-5.346e-5	1	NC	1	NC	1
427		5	max	.002	3	-.003	15	.048	4	1.813e-3	4	NC	1	NC	1
428			min	-.002	2	-.012	4	0	1	-7.159e-5	1	8614.44	4	NC	1
429		6	max	.002	3	-.004	15	.058	4	2.391e-3	4	NC	2	NC	1
430			min	-.002	2	-.015	4	0	1	-8.971e-5	1	6967.36	4	NC	1
431		7	max	.003	3	-.004	15	.067	4	2.969e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.018	4	0	1	-1.078e-4	1	5976.007	4	NC	1
433		8	max	.003	3	-.005	15	.076	4	3.546e-3	4	NC	5	NC	1
434			min	-.003	2	-.02	4	0	1	-1.26e-4	1	5364.401	4	NC	1
435		9	max	.004	3	-.005	15	.084	4	4.124e-3	4	NC	5	NC	1
436			min	-.003	2	-.021	4	0	1	-1.441e-4	1	5002.682	4	NC	1
437		10	max	.004	3	-.005	15	.093	4	4.702e-3	4	NC	5	NC	1
438			min	-.004	2	-.022	4	0	1	-1.622e-4	1	4827.81	4	NC	1
439		11	max	.005	3	-.005	15	.1	4	5.28e-3	4	NC	5	NC	1
440			min	-.004	2	-.022	4	-.001	1	-1.803e-4	1	4814.015	4	NC	1
441		12	max	.005	3	-.005	15	.108	4	5.857e-3	4	NC	5	NC	1
442			min	-.004	2	-.021	4	-.002	1	-1.985e-4	1	4962.912	4	NC	1
443		13	max	.006	3	-.005	15	.116	4	6.435e-3	4	NC	5	NC	1
444			min	-.005	2	-.02	4	-.002	1	-2.166e-4	1	5305.387	4	NC	1
445		14	max	.006	3	-.005	15	.124	4	7.013e-3	4	NC	5	NC	1
446			min	-.005	2	-.018	4	-.003	1	-2.347e-4	1	5917.617	4	NC	1
447		15	max	.007	3	-.004	15	.133	4	7.59e-3	4	NC	3	NC	1
448			min	-.006	2	-.016	4	-.003	1	-2.528e-4	1	6968.485	4	NC	1
449		16	max	.007	3	-.003	15	.142	4	8.168e-3	4	NC	1	NC	1
450			min	-.006	2	-.013	4	-.004	1	-2.71e-4	1	8867.043	4	NC	1
451		17	max	.008	3	-.002	15	.152	4	8.746e-3	4	NC	1	NC	1
452			min	-.006	2	-.009	4	-.004	1	-2.891e-4	1	NC	1	NC	1
453		18	max	.008	3	-.002	15	.163	4	9.323e-3	4	NC	1	NC	1
454			min	-.007	2	-.005	4	-.005	1	-3.072e-4	1	NC	1	NC	1
455		19	max	.009	3	0	10	.175	4	9.901e-3	4	NC	1	NC	1
456			min	-.007	2	-.002	1	-.006	1	-3.253e-4	1	NC	1	NC	1
457	M12	1	max	.002	1	.007	2	.006	1	-7.173e-6	10	NC	1	NC	2
458			min	0	3	-.009	3	-.175	4	-1.706e-4	4	NC	1	141.77	4
459		2	max	.002	1	.006	2	.006	1	-7.173e-6	10	NC	1	NC	2
460			min	0	3	-.008	3	-.161	4	-1.706e-4	4	NC	1	154.205	4
461		3	max	.002	1	.006	2	.005	1	-7.173e-6	10	NC	1	NC	2
462			min	0	3	-.008	3	-.147	4	-1.706e-4	4	NC	1	169.001	4
463		4	max	.002	1	.006	2	.005	1	-7.173e-6	10	NC	1	NC	2
464			min	0	3	-.007	3	-.133	4	-1.706e-4	4	NC	1	186.771	4
465		5	max	.002	1	.005	2	.004	1	-7.173e-6	10	NC	1	NC	2
466			min	0	3	-.007	3	-.119	4	-1.706e-4	4	NC	1	208.35	4
467		6	max	.002	1	.005	2	.004	1	-7.173e-6	10	NC	1	NC	2
468			min	0	3	-.006	3	-.106	4	-1.706e-4	4	NC	1	234.894	4
469		7	max	.002	1	.005	2	.003	1	-7.173e-6	10	NC	1	NC	2
470			min	0	3	-.006	3	-.093	4	-1.706e-4	4	NC	1	268.044	4
471		8	max	.002	1	.004	2	.003	1	-7.173e-6	10	NC	1	NC	2
472			min	0	3	-.005	3	-.08	4	-1.706e-4	4	NC	1	310.196	4
473		9	max	.001	1	.004	2	.002	1	-7.173e-6	10	NC	1	NC	1
474			min	0	3	-.005	3	-.068	4	-1.706e-4	4	NC	1	364.953	4
475		10	max	.001	1	.003	2	.002	1	-7.173e-6	10	NC	1	NC	1
476			min	0	3	-.004	3	-.057	4	-1.706e-4	4	NC	1	437.951	4
477		11	max	.001	1	.003	2	.002	1	-7.173e-6	10	NC	1	NC	1
478			min	0	3	-.004	3	-.046	4	-1.706e-4	4	NC	1	538.407	4
479		12	max	0	1	.003	2	.001	1	-7.173e-6	10	NC	1	NC	1
480			min	0	3	-.003	3	-.036	4	-1.706e-4	4	NC	1	682.267	4
481		13	max	0	1	.002	2	0	1	-7.173e-6	10	NC	1	NC	1
482			min	0	3	-.003	3	-.028	4	-1.706e-4	4	NC	1	899.125	4
483		14	max	0	1	.002	2	0	1	-7.173e-6	10	NC	1	NC	1
484			min	0	3	-.002	3	-.02	4	-1.706e-4	4	NC	1	1249.19	4
485		15	max	0	1	.002	2	0	1	-7.173e-6	10	NC	1	NC	1
486			min	0	3	-.002	3	-.013	4	-1.706e-4	4	NC	1	1871.25	4
487		16	max	0	1	.001	2	0	1	-7.173e-6	10	NC	1	NC	1
488			min	0	3	-.001	3	-.008	4	-1.706e-4	4	NC	1	3148.959	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-7.173e-6	10	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-1.706e-4	4	NC	1	6504.746	4
491		18	max	0	1	0	2	0	1	-7.173e-6	10	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-1.706e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-7.173e-6	10	NC	1	NC	1
494			min	0	1	0	1	0	1	-1.706e-4	4	NC	1	NC	1
495	M1	1	max	.011	3	.226	2	.619	4	6.618e-3	2	NC	1	NC	1
496			min	-.007	2	-.062	3	0	10	-1.645e-2	3	NC	1	NC	1
497		2	max	.011	3	.11	2	.601	4	6.748e-3	4	NC	5	NC	1
498			min	-.007	2	-.03	3	-.005	1	-8.168e-3	3	1169.349	2	NC	1
499		3	max	.011	3	.016	3	.582	4	1.227e-2	4	NC	5	NC	1
500			min	-.007	2	-.013	2	-.007	1	-1.23e-4	1	566.734	2	6803.685	5
501		4	max	.011	3	.087	3	.562	4	1.055e-2	4	NC	15	NC	1
502			min	-.007	2	-.149	2	-.006	1	-4.013e-3	3	361.088	2	4931.924	5
503		5	max	.011	3	.175	3	.542	4	8.842e-3	4	NC	15	NC	1
504			min	-.006	2	-.29	2	-.004	1	-7.928e-3	3	262.556	2	3980.005	5
505		6	max	.01	3	.268	3	.52	4	1.193e-2	2	8664.676	15	NC	1
506			min	-.006	2	-.425	2	-.002	1	-1.184e-2	3	207.969	2	3391.892	5
507		7	max	.01	3	.357	3	.499	4	1.59e-2	2	7333.042	15	NC	1
508			min	-.006	2	-.545	2	0	3	-1.576e-2	3	175.606	2	2965.343	4
509		8	max	.01	3	.431	3	.477	4	1.988e-2	2	6543.223	15	NC	1
510			min	-.006	2	-.64	2	0	12	-1.967e-2	3	156.406	2	2632.457	4
511		9	max	.01	3	.479	3	.455	4	2.218e-2	2	6129.048	15	NC	1
512			min	-.006	2	-.7	2	0	1	-2.023e-2	3	146.379	2	2396.047	4
513		10	max	.009	3	.497	3	.429	4	2.335e-2	2	6002.195	15	NC	1
514			min	-.006	2	-.72	2	0	10	-1.854e-2	3	143.436	2	2309.788	4
515		11	max	.009	3	.485	3	.401	4	2.453e-2	2	6128.691	15	NC	1
516			min	-.006	2	-.699	2	0	10	-1.685e-2	3	146.871	2	2330.001	4
517		12	max	.009	3	.445	3	.371	4	2.338e-2	2	6542.372	15	NC	1
518			min	-.006	2	-.637	2	0	1	-1.467e-2	3	157.839	2	2454.306	4
519		13	max	.009	3	.38	3	.335	4	1.875e-2	2	7331.387	15	NC	1
520			min	-.006	2	-.538	2	0	1	-1.173e-2	3	178.968	2	2880.316	4
521		14	max	.008	3	.296	3	.296	4	1.411e-2	2	8661.66	15	NC	1
522			min	-.005	2	-.414	2	0	12	-8.803e-3	3	214.967	2	3867.007	4
523		15	max	.008	3	.201	3	.255	4	9.48e-3	2	NC	15	NC	1
524			min	-.005	2	-.276	2	0	12	-5.872e-3	3	276.613	2	6266.332	4
525		16	max	.008	3	.102	3	.215	4	8.158e-3	4	NC	15	NC	1
526			min	-.005	2	-.137	2	0	12	-2.94e-3	3	389.951	2	NC	1
527		17	max	.008	3	.006	3	.179	4	9.327e-3	4	NC	5	NC	1
528			min	-.005	2	-.007	2	0	12	-9.039e-6	3	630.189	2	NC	1
529		18	max	.008	3	.103	2	.149	4	5.589e-3	2	NC	5	NC	1
530			min	-.005	2	-.083	3	0	12	-1.801e-3	3	1328.317	2	NC	1
531		19	max	.008	3	.203	2	.124	4	1.114e-2	2	NC	1	NC	1
532			min	-.005	2	-.166	3	0	1	-3.678e-3	3	NC	1	NC	1
533	M5	1	max	.032	3	.357	2	.619	4	0	1	NC	1	NC	1
534			min	-.023	2	.001	3	0	1	-1.221e-5	4	NC	1	NC	1
535		2	max	.032	3	.173	2	.605	4	6.274e-3	4	NC	5	NC	1
536			min	-.023	2	.002	3	0	1	0	1	747.256	2	9418.593	4
537		3	max	.032	3	.048	3	.588	4	1.241e-2	4	NC	5	NC	1
538			min	-.023	2	-.037	2	0	1	0	1	347.207	2	5555.522	4
539		4	max	.032	3	.174	3	.567	4	1.011e-2	4	9748.546	15	NC	1
540			min	-.022	2	-.295	2	0	1	0	1	209.327	2	4310.793	4
541		5	max	.031	3	.359	3	.545	4	7.812e-3	4	6757.656	15	NC	1
542			min	-.022	2	-.58	2	0	1	0	1	145.458	2	3706.481	4
543		6	max	.03	3	.571	3	.522	4	5.514e-3	4	5166.967	15	NC	1
544			min	-.021	2	-.866	2	0	1	0	1	111.363	2	3322.766	4
545		7	max	.03	3	.781	3	.498	4	3.215e-3	4	4254.912	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.021	2	-1.127	2	0	1	0	1	91.756	2	3001.933	4
547		8	max	.029	3	.959	3	.476	4	9.166e-4	4	3727.586	15	NC	1
548			min	-.021	2	-1.337	2	0	1	0	1	80.395	2	2672.571	4
549		9	max	.028	3	1.074	3	.455	4	0	1	3457.946	15	NC	1
550			min	-.02	2	-1.471	2	0	1	-7.183e-6	5	74.579	2	2388.008	4
551		10	max	.028	3	1.116	3	.429	4	0	1	3376.772	15	NC	1
552			min	-.02	2	-1.517	2	0	1	-6.887e-6	5	72.88	2	2331.729	4
553		11	max	.027	3	1.088	3	.401	4	0	1	3458.201	15	NC	1
554			min	-.02	2	-1.472	2	0	1	-6.591e-6	5	74.861	2	2366.611	4
555		12	max	.026	3	.992	3	.372	4	6.601e-4	4	3728.18	15	NC	1
556			min	-.019	2	-1.333	2	0	1	0	1	81.336	2	2406.223	4
557		13	max	.026	3	.838	3	.337	4	2.316e-3	4	4256.081	15	NC	1
558			min	-.019	2	-1.111	2	0	1	0	1	94.241	2	2798.154	4
559		14	max	.025	3	.645	3	.296	4	3.972e-3	4	5169.191	15	NC	1
560			min	-.019	2	-.837	2	0	1	0	1	117.09	2	3911.017	4
561		15	max	.024	3	.43	3	.252	4	5.628e-3	4	6761.98	15	NC	1
562			min	-.018	2	-.543	2	0	1	0	1	158.287	2	7307.37	4
563		16	max	.023	3	.214	3	.21	4	7.284e-3	4	9757.542	15	NC	1
564			min	-.018	2	-.26	2	0	1	0	1	239.373	2	NC	1
565		17	max	.023	3	.015	3	.173	4	8.94e-3	4	NC	5	NC	1
566			min	-.018	2	-.019	2	0	1	0	1	424.503	2	NC	1
567		18	max	.023	3	.154	2	.144	4	4.521e-3	4	NC	5	NC	1
568			min	-.018	2	-.151	3	0	1	0	1	965.062	2	NC	1
569		19	max	.023	3	.289	2	.124	4	0	1	NC	1	NC	1
570			min	-.017	2	-.299	3	0	1	-6.625e-6	4	NC	1	NC	1
571	M9	1	max	.011	3	.226	2	.619	4	1.645e-2	3	NC	1	NC	1
572			min	-.007	2	-.062	3	0	1	-6.618e-3	2	NC	1	NC	1
573		2	max	.011	3	.11	2	.604	4	8.168e-3	3	NC	5	NC	1
574			min	-.007	2	-.03	3	0	10	-3.246e-3	2	1169.349	2	NC	1
575		3	max	.011	3	.016	3	.586	4	1.236e-2	4	NC	5	NC	1
576			min	-.007	2	-.013	2	0	10	-2.533e-5	10	566.734	2	6005.513	4
577		4	max	.011	3	.087	3	.566	4	9.77e-3	5	NC	15	NC	1
578			min	-.007	2	-.149	2	0	10	-3.979e-3	2	361.088	2	4524.001	4
579		5	max	.011	3	.175	3	.544	4	7.928e-3	3	NC	15	NC	1
580			min	-.006	2	-.29	2	0	10	-7.954e-3	2	262.556	2	3782.66	4
581		6	max	.01	3	.268	3	.522	4	1.184e-2	3	8625.323	15	NC	1
582			min	-.006	2	-.425	2	0	10	-1.193e-2	2	207.969	2	3317.588	4
583		7	max	.01	3	.357	3	.499	4	1.576e-2	3	7300.594	15	NC	1
584			min	-.006	2	-.545	2	0	1	-1.59e-2	2	175.606	2	2964.224	4
585		8	max	.01	3	.431	3	.477	4	1.967e-2	3	6514.783	15	NC	1
586			min	-.006	2	-.64	2	0	1	-1.988e-2	2	156.406	2	2648.98	4
587		9	max	.01	3	.479	3	.455	4	2.023e-2	3	6102.652	15	NC	1
588			min	-.006	2	-.7	2	0	10	-2.218e-2	2	146.379	2	2389.407	4
589		10	max	.009	3	.497	3	.429	4	1.854e-2	3	5976.369	15	NC	1
590			min	-.006	2	-.72	2	0	1	-2.335e-2	2	143.436	2	2310.618	4
591		11	max	.009	3	.485	3	.401	4	1.685e-2	3	6102.208	15	NC	1
592			min	-.006	2	-.699	2	0	1	-2.453e-2	2	146.871	2	2337.507	4
593		12	max	.009	3	.445	3	.371	4	1.467e-2	3	6513.901	15	NC	1
594			min	-.006	2	-.637	2	0	10	-2.338e-2	2	157.839	2	2439.73	4
595		13	max	.009	3	.38	3	.335	4	1.173e-2	3	7299.166	15	NC	1
596			min	-.006	2	-.538	2	0	10	-1.875e-2	2	178.968	2	2877.102	4
597		14	max	.008	3	.296	3	.295	4	8.803e-3	3	8623.076	15	NC	1
598			min	-.005	2	-.414	2	-.002	1	-1.411e-2	2	214.967	2	3949.301	5
599		15	max	.008	3	.201	3	.253	4	5.872e-3	3	NC	15	NC	1
600			min	-.005	2	-.276	2	-.004	1	-9.48e-3	2	276.613	2	6718.195	5
601		16	max	.008	3	.102	3	.212	4	7.26e-3	5	NC	15	NC	1
602			min	-.005	2	-.137	2	-.006	1	-4.847e-3	2	389.951	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.008	3	.006	3	.176	4	9.101e-3	4	NC	5	NC	1
604		min	-.005	2	-.007	2	-.006	1	-4.288e-4	1	630.189	2	NC	1
605	18	max	.008	3	.103	2	.146	4	4.467e-3	5	NC	5	NC	1
606		min	-.005	2	-.083	3	-.005	1	-5.589e-3	2	1328.317	2	NC	1
607	19	max	.008	3	.203	2	.124	4	3.678e-3	3	NC	1	NC	1
608		min	-.005	2	-.166	3	0	12	-1.114e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

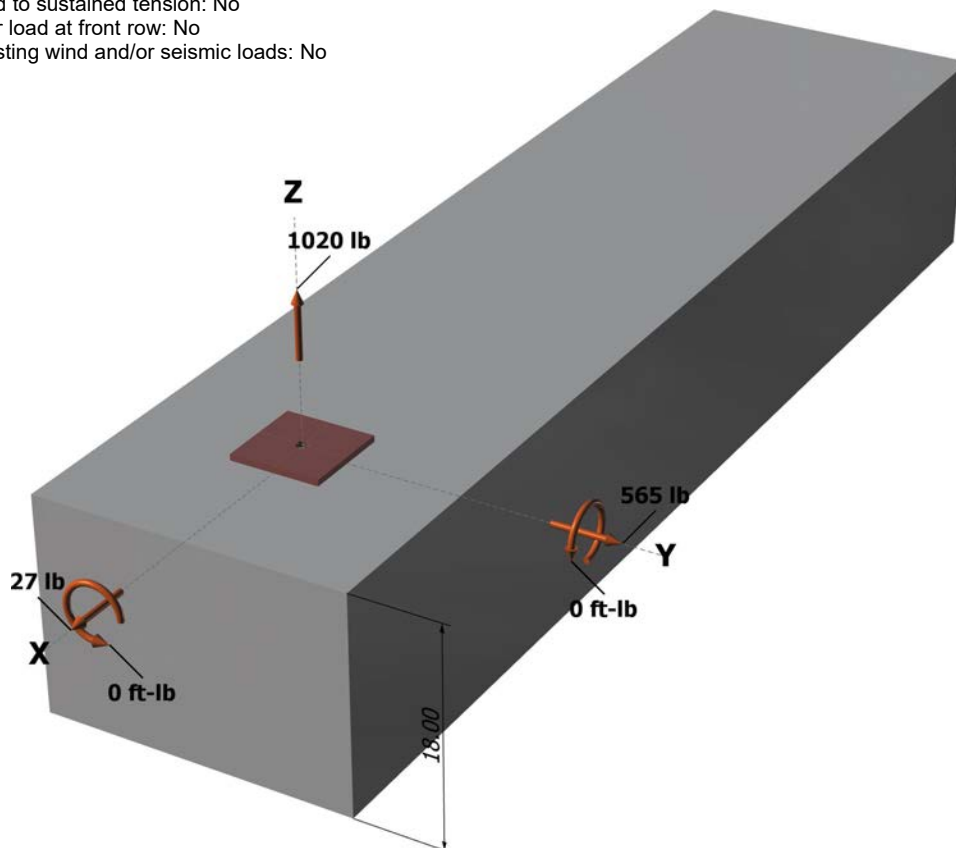
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.6025.0

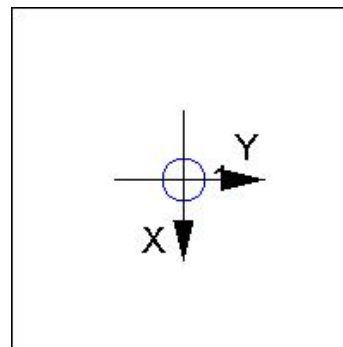
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

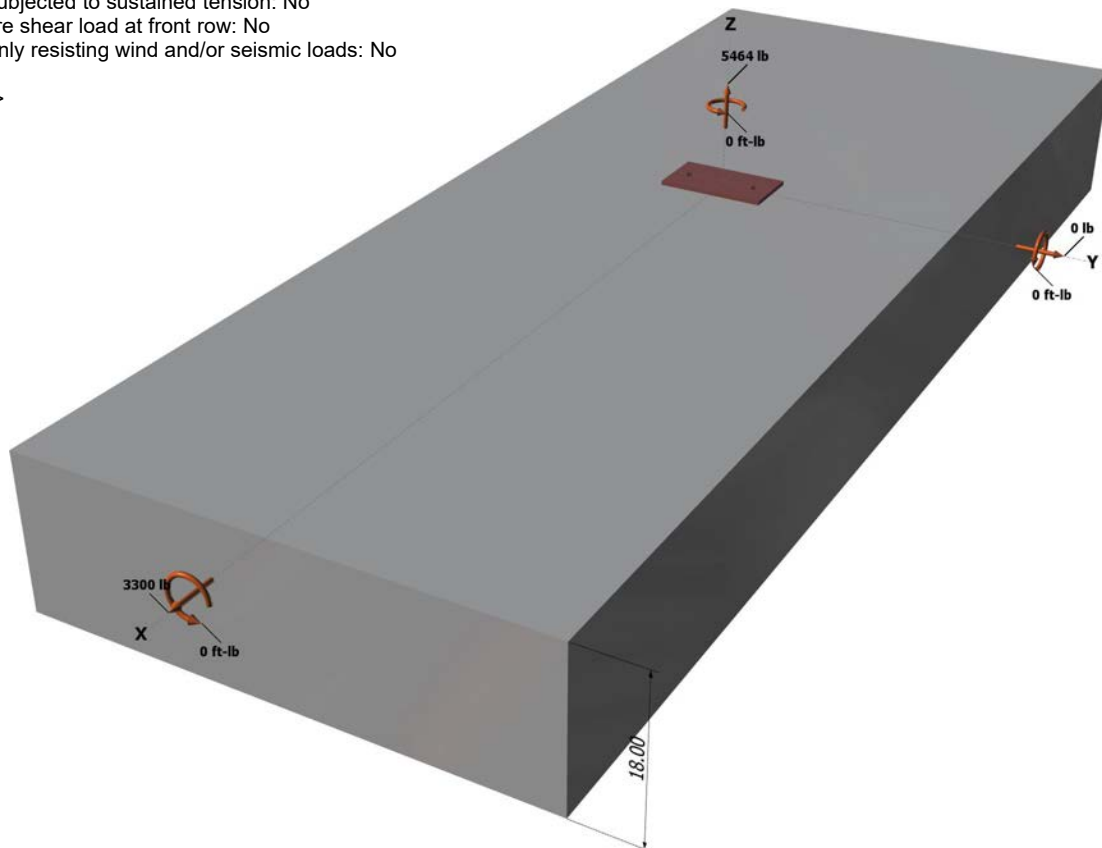
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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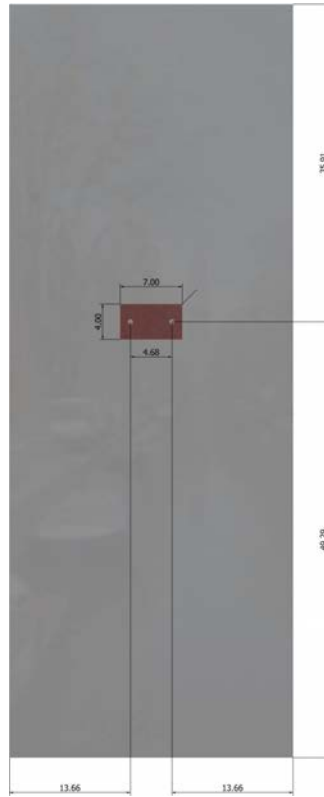




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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

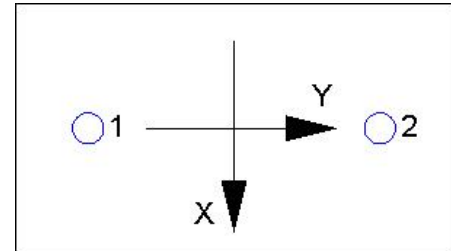
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.