

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

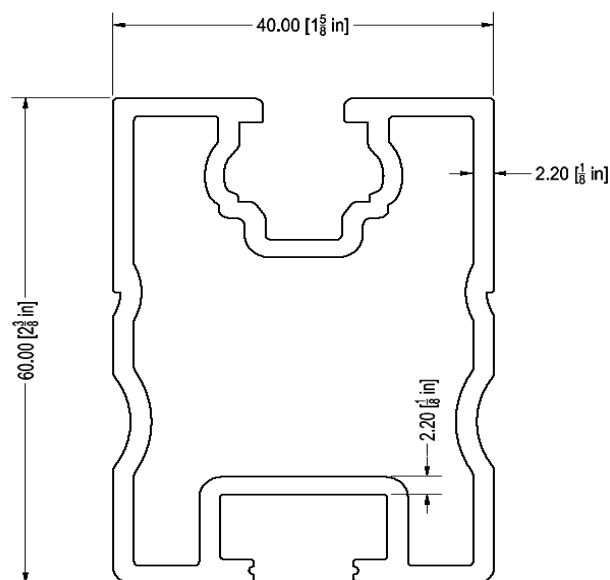
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

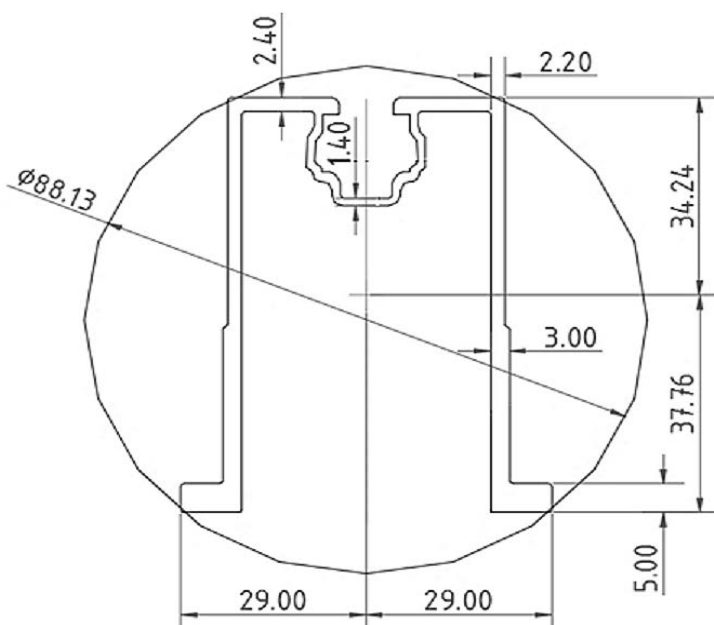
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	<u>48</u> in
ΦF_{ty} STRONG-AXIS =	29.75 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.407 k-ft
M_z =	0.024 k-ft
$M_{y \text{ allowable}}$ =	1.266 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	35%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

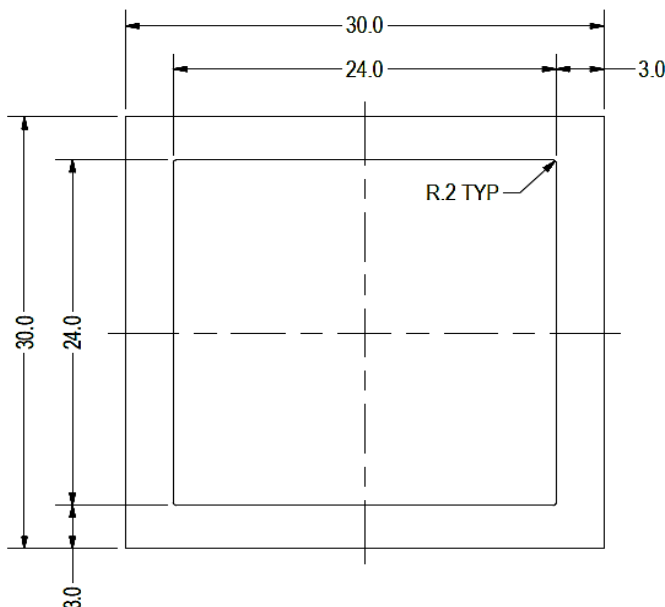
Girder Type =	<u>Flex Profi</u>
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	<u>33.78</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	14.29 ksi
$\Phi F_{ty \text{ STRONG-AXIS}}$ =	29.88 ksi
$\Phi F_{ty \text{ WEAK-AXIS}}$ =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.473 k-ft
M_z =	-0.015 k-ft
P_n =	-0.043 k
$M_{y \text{ allowable}}$ =	1.466 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	29.696 k
Utilization =	35%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

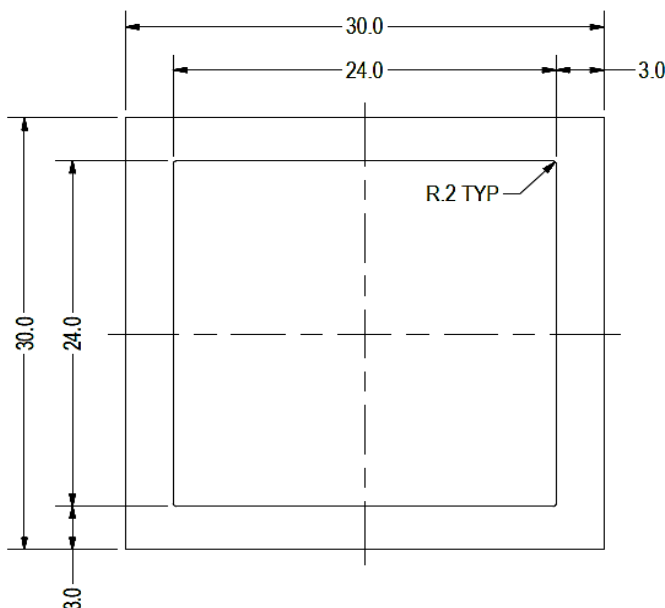
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.062 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.188 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	5%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.794 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	10%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.158 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

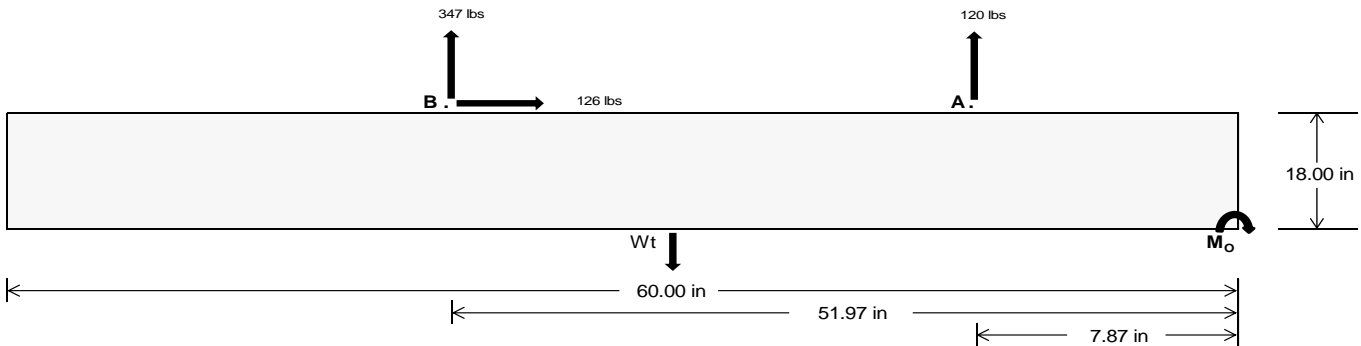
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	523.21	1508.40	k
Compressive Load =	1380.22	985.57	k
Lateral Load =	21.37	547.66	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21265.0$ in-lbs
Resisting Force Required = 708.83 lbs
S.F. = 1.67
Weight Required = 1181.39 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 126.37 lbs
Friction = 0.4
Weight Required = 315.94 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 126.37 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	419 lbs	419 lbs	419 lbs	419 lbs	546 lbs	546 lbs	546 lbs	546 lbs	694 lbs	694 lbs	694 lbs	694 lbs	-239 lbs	-239 lbs	-239 lbs	-239 lbs
F_B	302 lbs	302 lbs	302 lbs	302 lbs	390 lbs	390 lbs	390 lbs	390 lbs	497 lbs	497 lbs	497 lbs	497 lbs	-695 lbs	-695 lbs	-695 lbs	-695 lbs
F_V	21 lbs	21 lbs	21 lbs	21 lbs	219 lbs	219 lbs	219 lbs	219 lbs	179 lbs	179 lbs	179 lbs	179 lbs	-253 lbs	-253 lbs	-253 lbs	-253 lbs
P_{total}	2624 lbs	2715 lbs	2805 lbs	2896 lbs	2839 lbs	2929 lbs	3020 lbs	3111 lbs	3094 lbs	3184 lbs	3275 lbs	3366 lbs	208 lbs	262 lbs	317 lbs	371 lbs
M	252 lbs-ft	252 lbs-ft	252 lbs-ft	252 lbs-ft	621 lbs-ft	621 lbs-ft	621 lbs-ft	621 lbs-ft	639 lbs-ft	639 lbs-ft	639 lbs-ft	639 lbs-ft	451 lbs-ft	451 lbs-ft	451 lbs-ft	451 lbs-ft
e	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.20 ft	0.19 ft	2.17 ft	1.72 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	265.3 psf	263.2 psf	261.2 psf	259.4 psf	239.2 psf	238.2 psf	237.3 psf	236.5 psf	265.9 psf	263.7 psf	261.7 psf	259.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	334.4 psf	329.1 psf	324.2 psf	319.8 psf	409.6 psf	400.9 psf	392.9 psf	385.6 psf	441.2 psf	431.0 psf	421.8 psf	413.3 psf	239.9 psf	122.4 psf	102.5 psf	96.4 psf

Maximum Bearing Pressure = 441 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

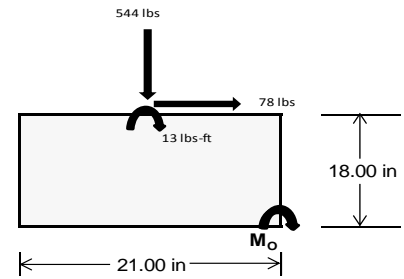
Overturning Check

$M_o = 346.5 \text{ ft-lbs}$
 Resisting Force Required = 396.04 lbs
 S.F. = 1.67
 Weight Required = 660.07 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	102 lbs	73 lbs	51 lbs	234 lbs	544 lbs	196 lbs	66 lbs	-17 lbs	17 lbs
F_v	12 lbs	103 lbs	12 lbs	9 lbs	78 lbs	9 lbs	12 lbs	103 lbs	12 lbs
P_{total}	2458 lbs	2429 lbs	2407 lbs	2477 lbs	2787 lbs	2438 lbs	755 lbs	672 lbs	706 lbs
M	34 lbs-ft	172 lbs-ft	35 lbs-ft	25 lbs-ft	129 lbs-ft	27 lbs-ft	34 lbs-ft	172 lbs-ft	35 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.26 ft	0.05 ft
$L/6$	0.29 ft	1.61 ft	1.72 ft	1.73 ft	1.66 ft	1.73 ft	1.66 ft	1.24 ft	1.65 ft
f_{min}	267.4 sqft	210.3 sqft	261.2 sqft	273.2 sqft	267.8 sqft	268.1 sqft	72.8 sqft	9.5 sqft	66.9 sqft
f_{max}	294.3 psf	345.0 psf	288.9 psf	293.0 psf	369.2 psf	289.2 psf	99.8 psf	144.1 psf	94.5 psf



Maximum Bearing Pressure = 369 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.773 k
Allowable Uplift =	1.214 k
Utilization =	<u>64%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.101 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.062 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Diagonal Strut

Maximum Axial Load =	0.188 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.142 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.158 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.053 in
	<u>0.053 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$124.989$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$129.794$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.266 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.41$$

$$20.702$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.41$$

$$24.5845$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b [Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.466 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \\\...\\PVMMini 60 Cell 1V 15° 160mph 30psf 4ft 7-10.r3dPage 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	252.268	1	.132	6	.121	1	0	10	0	4	0	15
30		min	-363.97	3	.029	15	-.416	5	0	4	0	3	0	6
31	16	max	252.364	1	.096	2	.121	1	0	10	0	4	0	15
32		min	-363.898	3	.02	15	-.503	5	0	4	0	3	0	6
33	17	max	252.461	1	.066	2	.121	1	0	10	0	4	0	15
34		min	-363.826	3	.011	15	-.591	5	0	4	0	3	0	6
35	18	max	252.557	1	.037	2	.121	1	0	10	0	14	0	15
36		min	-363.753	3	.002	9	-.678	5	0	4	0	3	0	6
37	19	max	252.654	1	.008	10	.121	1	0	10	0	1	0	15
38		min	-363.681	3	-.023	14	-.765	5	0	4	0	3	0	6
39	M3	1	max	56.05	2	1.812	.004	10	0	5	0	4	0	6
40		min	-49.447	14	.425	15	-1.346	4	0	1	0	10	0	15
41	2	max	55.983	2	1.634	6	.004	10	0	5	0	1	0	6
42		min	-49.513	14	.383	15	-1.212	4	0	1	0	10	0	15
43	3	max	55.916	2	1.456	6	.004	10	0	5	0	1	0	2
44		min	-49.579	14	.341	15	-1.079	4	0	1	0	10	0	15
45	4	max	55.848	2	1.278	6	.004	10	0	5	0	1	0	15
46		min	-49.645	14	.299	15	-.945	4	0	1	0	5	0	4
47	5	max	55.781	2	1.1	6	.004	10	0	5	0	1	0	15
48		min	-49.711	14	.257	15	-.812	4	0	1	0	5	0	4
49	6	max	55.714	2	.922	6	.004	10	0	5	0	1	0	15
50		min	-49.777	14	.215	15	-.678	4	0	1	0	5	0	4
51	7	max	55.647	2	.744	6	.004	10	0	5	0	1	0	15
52		min	-49.843	14	.173	15	-.545	4	0	1	0	5	0	4
53	8	max	55.58	2	.566	6	.004	10	0	5	0	1	0	15
54		min	-49.909	14	.132	15	-.411	4	0	1	0	5	0	4
55	9	max	55.513	2	.388	6	.004	10	0	5	0	1	0	15
56		min	-49.975	14	.09	15	-.277	4	0	1	0	5	-.001	4
57	10	max	55.446	2	.21	6	.004	10	0	5	0	1	0	15
58		min	-50.041	14	.048	15	-.144	4	0	1	0	5	-.001	4
59	11	max	55.379	2	.04	2	.019	5	0	5	0	1	0	15
60		min	-50.107	14	.006	15	-.138	1	0	1	0	5	-.001	4
61	12	max	55.312	2	-.036	15	.152	5	0	5	0	1	0	15
62		min	-50.172	14	-.146	4	-.138	1	0	1	0	5	-.001	4
63	13	max	55.245	2	-.078	15	.286	5	0	5	0	1	0	15
64		min	-50.238	14	-.324	4	-.138	1	0	1	0	5	-.001	4
65	14	max	55.177	2	-.119	15	.42	5	0	5	0	9	0	15
66		min	-50.304	14	-.502	4	-.138	1	0	1	0	5	-.001	4
67	15	max	55.11	2	-.161	15	.553	5	0	5	0	10	0	15
68		min	-50.37	14	-.68	4	-.138	1	0	1	0	4	0	4
69	16	max	55.043	2	-.203	15	.687	5	0	5	0	10	0	15
70		min	-50.436	14	-.858	4	-.138	1	0	1	0	4	0	4
71	17	max	54.976	2	-.245	15	.82	5	0	5	0	10	0	15
72		min	-50.502	14	-1.036	4	-.138	1	0	1	0	4	0	4
73	18	max	54.909	2	-.287	15	.954	5	0	5	0	10	0	15
74		min	-50.568	14	-1.214	4	-.138	1	0	1	0	4	0	4
75	19	max	54.842	2	-.329	15	1.087	5	0	5	0	5	0	1
76		min	-50.634	14	-1.392	4	-.138	1	0	1	0	1	0	1
77	M4	1	max	347.718	1	0	.022	10	0	1	0	5	0	1
78		min	-119.242	3	0	1	-15.223	4	0	1	0	2	0	1
79	2	max	347.783	1	0	1	.022	10	0	1	0	10	0	1
80		min	-119.194	3	0	1	-15.279	4	0	1	-.001	4	0	1
81	3	max	347.848	1	0	1	.022	10	0	1	0	10	0	1
82		min	-119.145	3	0	1	-15.335	4	0	1	-.003	4	0	1
83	4	max	347.912	1	0	1	.022	10	0	1	0	10	0	1
84		min	-119.097	3	0	1	-15.391	4	0	1	-.004	4	0	1
85	5	max	347.977	1	0	1	.022	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-119.048	3	0	1	-15.447	4	0	1	-.005	4	0	1
87		6	max	348.042	1	0	1	.022	10	0	1	0	10	0	1
88			min	-119	3	0	1	-15.503	4	0	1	-.007	4	0	1
89		7	max	348.106	1	0	1	.022	10	0	1	0	10	0	1
90			min	-118.951	3	0	1	-15.559	4	0	1	-.008	4	0	1
91		8	max	348.171	1	0	1	.022	10	0	1	0	10	0	1
92			min	-118.903	3	0	1	-15.615	4	0	1	-.01	4	0	1
93		9	max	348.236	1	0	1	.022	10	0	1	0	10	0	1
94			min	-118.854	3	0	1	-15.671	4	0	1	-.011	4	0	1
95		10	max	348.3	1	0	1	.022	10	0	1	0	10	0	1
96			min	-118.806	3	0	1	-15.727	4	0	1	-.012	4	0	1
97		11	max	348.365	1	0	1	.022	10	0	1	0	10	0	1
98			min	-118.757	3	0	1	-15.784	4	0	1	-.014	4	0	1
99		12	max	348.43	1	0	1	.022	10	0	1	0	10	0	1
100			min	-118.709	3	0	1	-15.84	4	0	1	-.015	4	0	1
101		13	max	348.495	1	0	1	.022	10	0	1	0	10	0	1
102			min	-118.66	3	0	1	-15.896	4	0	1	-.017	4	0	1
103		14	max	348.559	1	0	1	.022	10	0	1	0	10	0	1
104			min	-118.612	3	0	1	-15.952	4	0	1	-.018	4	0	1
105		15	max	348.624	1	0	1	.022	10	0	1	0	10	0	1
106			min	-118.563	3	0	1	-16.008	4	0	1	-.02	4	0	1
107		16	max	348.689	1	0	1	.022	10	0	1	0	10	0	1
108			min	-118.515	3	0	1	-16.064	4	0	1	-.021	4	0	1
109		17	max	348.753	1	0	1	.022	10	0	1	0	10	0	1
110			min	-118.466	3	0	1	-16.12	4	0	1	-.022	4	0	1
111		18	max	348.818	1	0	1	.022	10	0	1	0	10	0	1
112			min	-118.417	3	0	1	-16.176	4	0	1	-.024	4	0	1
113		19	max	348.883	1	0	1	.022	10	0	1	0	10	0	1
114			min	-118.369	3	0	1	-16.232	4	0	1	-.025	4	0	1
115	M6	1	max	792.418	1	.649	6	.838	4	0	3	0	3	0	1
116			min	-1141.887	3	.15	15	-.289	3	0	5	0	1	0	1
117		2	max	792.514	1	.611	6	.75	4	0	3	0	4	0	15
118			min	-1141.815	3	.141	15	-.289	3	0	5	0	1	0	6
119		3	max	792.61	1	.574	6	.663	4	0	3	0	4	0	15
120			min	-1141.743	3	.132	15	-.289	3	0	5	0	2	0	6
121		4	max	792.707	1	.536	6	.576	4	0	3	0	4	0	15
122			min	-1141.671	3	.124	15	-.289	3	0	5	0	2	0	6
123		5	max	792.803	1	.498	6	.488	4	0	3	0	4	0	15
124			min	-1141.598	3	.115	15	-.289	3	0	5	0	3	0	6
125		6	max	792.899	1	.46	6	.401	4	0	3	0	4	0	15
126			min	-1141.526	3	.106	15	-.289	3	0	5	0	3	0	6
127		7	max	792.996	1	.422	6	.314	4	0	3	0	4	0	15
128			min	-1141.454	3	.097	15	-.289	3	0	5	0	3	0	6
129		8	max	793.092	1	.384	6	.226	4	0	3	0	4	0	15
130			min	-1141.381	3	.088	15	-.289	3	0	5	0	3	0	6
131		9	max	793.189	1	.347	6	.139	4	0	3	0	4	0	15
132			min	-1141.309	3	.079	15	-.289	3	0	5	0	3	0	6
133		10	max	793.285	1	.309	6	.052	4	0	3	0	4	0	15
134			min	-1141.237	3	.07	15	-.289	3	0	5	0	3	0	6
135		11	max	793.381	1	.271	6	.031	9	0	3	0	4	0	15
136			min	-1141.165	3	.061	15	-.289	3	0	5	0	3	0	6
137		12	max	793.478	1	.241	2	.031	9	0	3	0	4	0	15
138			min	-1141.092	3	.052	15	-.289	3	0	5	0	3	0	6
139		13	max	793.574	1	.212	2	.031	9	0	3	0	4	0	15
140			min	-1141.02	3	.044	15	-.289	3	0	5	0	3	0	6
141		14	max	793.67	1	.182	2	.031	9	0	3	0	4	0	15
142			min	-1140.948	3	.035	15	-.31	5	0	5	0	3	0	6



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	793.767	1	.153	2	.031	9	0	3	0	4	0	15
144		min	-1140.875	3	.026	15	-.397	5	0	5	0	3	0	6
145	16	max	793.863	1	.123	2	.031	9	0	3	0	4	0	15
146		min	-1140.803	3	.017	15	-.484	5	0	5	0	3	0	6
147	17	max	793.959	1	.094	2	.031	9	0	3	0	4	0	15
148		min	-1140.731	3	.002	9	-.572	5	0	5	0	3	0	6
149	18	max	794.056	1	.064	2	.031	9	0	3	0	4	0	15
150		min	-1140.659	3	-.023	9	-.659	5	0	5	0	3	0	6
151	19	max	794.152	1	.035	2	.031	9	0	3	0	4	0	15
152		min	-1140.586	3	-.047	9	-.746	5	0	5	0	3	0	6
153	M7	1	max	187.722	2	1.819	4	0	2	0	9	0	4	4
154		min	-91.09	9	.431	15	-1.41	4	0	3	0	3	0	15
155	2	max	187.655	2	1.641	4	0	2	0	9	0	4	0	4
156		min	-91.146	9	.389	15	-1.277	4	0	3	0	3	0	15
157	3	max	187.588	2	1.463	4	0	2	0	9	0	4	0	2
158		min	-91.201	9	.348	15	-1.143	4	0	3	0	3	0	9
159	4	max	187.521	2	1.285	4	0	2	0	9	0	1	0	15
160		min	-91.257	9	.306	15	-1.01	4	0	3	0	3	0	9
161	5	max	187.454	2	1.107	4	0	2	0	9	0	1	0	15
162		min	-91.313	9	.264	15	-.876	4	0	3	0	3	0	6
163	6	max	187.387	2	.929	4	0	2	0	9	0	1	0	15
164		min	-91.369	9	.222	15	-.743	4	0	3	0	5	0	6
165	7	max	187.32	2	.751	4	0	2	0	9	0	1	0	15
166		min	-91.425	9	.18	15	-.609	4	0	3	0	5	0	6
167	8	max	187.252	2	.573	4	0	2	0	9	0	1	0	15
168		min	-91.481	9	.138	15	-.475	4	0	3	0	5	0	6
169	9	max	187.185	2	.395	4	0	2	0	9	0	1	0	15
170		min	-91.537	9	.096	15	-.342	4	0	3	0	5	-.001	6
171	10	max	187.118	2	.217	4	0	2	0	9	0	1	0	15
172		min	-91.593	9	.055	15	-.208	4	0	3	0	5	-.001	6
173	11	max	187.051	2	.057	2	0	2	0	9	0	1	0	15
174		min	-91.649	9	.001	9	-.075	4	0	3	0	5	-.001	6
175	12	max	186.984	2	-.029	15	.061	5	0	9	0	1	0	15
176		min	-91.705	9	-.14	6	-.018	1	0	3	0	5	-.001	6
177	13	max	186.917	2	-.071	15	.195	5	0	9	0	1	0	15
178		min	-91.761	9	-.318	6	-.018	1	0	3	0	5	-.001	6
179	14	max	186.85	2	-.113	15	.328	5	0	9	0	1	0	15
180		min	-91.816	9	-.496	6	-.018	1	0	3	0	5	-.001	6
181	15	max	186.783	2	-.155	15	.462	5	0	9	0	1	0	15
182		min	-91.872	9	-.674	6	-.018	1	0	3	0	5	0	6
183	16	max	186.716	2	-.196	15	.595	5	0	9	0	1	0	15
184		min	-91.928	9	-.852	6	-.018	1	0	3	0	5	0	6
185	17	max	186.649	2	-.238	15	.729	5	0	9	0	9	0	15
186		min	-91.984	9	-1.03	6	-.018	1	0	3	0	5	0	6
187	18	max	186.582	2	-.28	15	.862	5	0	9	0	9	0	15
188		min	-92.04	9	-1.208	6	-.018	1	0	3	0	5	0	6
189	19	max	186.514	2	-.322	15	.996	5	0	9	0	9	0	1
190		min	-92.096	9	-1.386	6	-.018	1	0	3	0	3	0	1
191	M8	1	max	1060.542	2	0	.091	9	0	1	0	4	0	1
192		min	-403.345	3	0	1	-15.581	4	0	1	0	3	0	1
193	2	max	1060.607	2	0	1	.091	9	0	1	0	9	0	1
194		min	-403.296	3	0	1	-15.637	4	0	1	-.001	4	0	1
195	3	max	1060.671	2	0	1	.091	9	0	1	0	9	0	1
196		min	-403.248	3	0	1	-15.693	4	0	1	-.003	4	0	1
197	4	max	1060.736	2	0	1	.091	9	0	1	0	9	0	1
198		min	-403.199	3	0	1	-15.749	4	0	1	-.004	4	0	1
199	5	max	1060.801	2	0	1	.091	9	0	1	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-403.151	3	0	1	-15.805	4	0	1	-.006	4	0	1
201		6	max	1060.865	2	0	1	.091	9	0	1	0	9	0	1
202			min	-403.102	3	0	1	-15.861	4	0	1	-.007	4	0	1
203		7	max	1060.93	2	0	1	.091	9	0	1	0	9	0	1
204			min	-403.054	3	0	1	-15.917	4	0	1	-.008	4	0	1
205		8	max	1060.995	2	0	1	.091	9	0	1	0	9	0	1
206			min	-403.005	3	0	1	-15.973	4	0	1	-.01	4	0	1
207		9	max	1061.059	2	0	1	.091	9	0	1	0	9	0	1
208			min	-402.957	3	0	1	-16.029	4	0	1	-.011	4	0	1
209		10	max	1061.124	2	0	1	.091	9	0	1	0	9	0	1
210			min	-402.908	3	0	1	-16.085	4	0	1	-.013	4	0	1
211		11	max	1061.189	2	0	1	.091	9	0	1	0	9	0	1
212			min	-402.86	3	0	1	-16.142	4	0	1	-.014	4	0	1
213		12	max	1061.254	2	0	1	.091	9	0	1	0	9	0	1
214			min	-402.811	3	0	1	-16.198	4	0	1	-.016	4	0	1
215		13	max	1061.318	2	0	1	.091	9	0	1	0	9	0	1
216			min	-402.763	3	0	1	-16.254	4	0	1	-.017	4	0	1
217		14	max	1061.383	2	0	1	.091	9	0	1	0	9	0	1
218			min	-402.714	3	0	1	-16.31	4	0	1	-.019	4	0	1
219		15	max	1061.448	2	0	1	.091	9	0	1	0	9	0	1
220			min	-402.666	3	0	1	-16.366	4	0	1	-.02	4	0	1
221		16	max	1061.512	2	0	1	.091	9	0	1	0	9	0	1
222			min	-402.617	3	0	1	-16.422	4	0	1	-.021	4	0	1
223		17	max	1061.577	2	0	1	.091	9	0	1	0	9	0	1
224			min	-402.568	3	0	1	-16.478	4	0	1	-.023	4	0	1
225		18	max	1061.642	2	0	1	.091	9	0	1	0	9	0	1
226			min	-402.52	3	0	1	-16.534	4	0	1	-.024	4	0	1
227		19	max	1061.707	2	0	1	.091	9	0	1	0	9	0	1
228			min	-402.471	3	0	1	-16.59	4	0	1	-.026	4	0	1
229	M10	1	max	252.241	1	.692	4	.967	5	0	1	0	4	0	1
230			min	-327.436	3	.174	15	-.105	1	-.001	5	0	3	0	1
231		2	max	252.338	1	.654	4	.88	5	0	1	0	4	0	15
232			min	-327.363	3	.165	15	-.105	1	-.001	5	0	3	0	4
233		3	max	252.434	1	.616	4	.792	5	0	1	0	4	0	15
234			min	-327.291	3	.156	15	-.105	1	-.001	5	0	3	0	4
235		4	max	252.53	1	.578	4	.705	5	0	1	0	4	0	15
236			min	-327.219	3	.147	15	-.105	1	-.001	5	0	3	0	4
237		5	max	252.627	1	.541	4	.618	5	0	1	0	5	0	15
238			min	-327.146	3	.138	15	-.105	1	-.001	5	0	3	0	4
239		6	max	252.723	1	.503	4	.53	5	0	1	0	5	0	15
240			min	-327.074	3	.129	15	-.105	1	-.001	5	0	3	0	4
241		7	max	252.82	1	.465	4	.443	5	0	1	0	5	0	15
242			min	-327.002	3	.12	15	-.105	1	-.001	5	0	3	0	4
243		8	max	252.916	1	.427	4	.356	5	0	1	0	5	0	15
244			min	-326.93	3	.111	15	-.105	1	-.001	5	0	3	0	4
245		9	max	253.012	1	.389	4	.268	5	0	1	0	5	0	15
246			min	-326.857	3	.102	15	-.105	1	-.001	5	0	3	0	4
247		10	max	253.109	1	.351	4	.181	5	0	1	0	5	0	15
248			min	-326.785	3	.094	15	-.105	1	-.001	5	0	3	0	4
249		11	max	253.205	1	.314	4	.094	5	0	1	0	5	0	15
250			min	-326.713	3	.085	15	-.105	1	-.001	5	0	3	0	4
251		12	max	253.301	1	.276	4	.006	5	0	1	0	5	0	15
252			min	-326.641	3	.076	15	-.105	1	-.001	5	0	3	0	4
253		13	max	253.398	1	.238	4	0	10	0	1	0	5	0	15
254			min	-326.568	3	.067	15	-.105	1	-.001	5	0	3	0	4
255		14	max	253.494	1	.2	4	0	10	0	1	0	5	0	15
256			min	-326.496	3	.058	15	-.179	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	253.59	1	.162	4	0	10	0	1	0	5	0	15
258		min	-326.424	3	.049	15	-.266	4	-.001	5	0	3	0	4
259	16	max	253.687	1	.124	4	0	10	0	1	0	5	0	15
260		min	-326.351	3	.04	15	-.353	4	-.001	5	0	3	0	4
261	17	max	253.783	1	.087	4	0	10	0	1	0	5	0	15
262		min	-326.279	3	.026	9	-.441	4	-.001	5	0	3	0	4
263	18	max	253.88	1	.051	3	0	10	0	1	0	5	0	15
264		min	-326.207	3	.002	9	-.528	4	-.001	5	0	3	0	4
265	19	max	253.976	1	.028	3	0	10	0	1	0	5	0	15
266		min	-326.135	3	-.023	9	-.615	4	-.001	5	0	3	0	4
267	M11	1	max	55.644	2	1.812	.144	1	0	4	0	5	0	6
268		min	-48.457	9	.424	15	-1.238	5	0	10	0	1	0	15
269	2	max	55.577	2	1.634	6	.144	1	0	4	0	5	0	6
270		min	-48.513	9	.382	15	-1.104	5	0	10	0	1	0	15
271	3	max	55.509	2	1.456	6	.144	1	0	4	0	5	0	2
272		min	-48.569	9	.34	15	-.971	5	0	10	0	1	0	3
273	4	max	55.442	2	1.278	6	.144	1	0	4	0	3	0	15
274		min	-48.625	9	.299	15	-.837	5	0	10	0	1	0	4
275	5	max	55.375	2	1.1	6	.144	1	0	4	0	3	0	15
276		min	-48.681	9	.257	15	-.704	5	0	10	0	1	0	4
277	6	max	55.308	2	.922	6	.144	1	0	4	0	3	0	15
278		min	-48.737	9	.215	15	-.57	5	0	10	0	1	0	4
279	7	max	55.241	2	.744	6	.144	1	0	4	0	3	0	15
280		min	-48.793	9	.173	15	-.437	5	0	10	0	1	0	4
281	8	max	55.174	2	.566	6	.144	1	0	4	0	3	0	15
282		min	-48.849	9	.131	15	-.303	5	0	10	0	4	0	4
283	9	max	55.107	2	.388	6	.144	1	0	4	0	3	0	15
284		min	-48.905	9	.089	15	-.17	5	0	10	0	4	-.001	4
285	10	max	55.04	2	.21	6	.144	1	0	4	0	3	0	15
286		min	-48.961	9	.047	15	-.036	5	0	10	0	4	-.001	4
287	11	max	54.973	2	.04	2	.144	1	0	4	0	3	0	15
288		min	-49.016	9	.003	3	-.024	3	0	10	0	4	-.001	4
289	12	max	54.906	2	-.036	15	.262	4	0	4	0	3	0	15
290		min	-49.072	9	-.147	4	-.024	3	0	10	0	4	-.001	4
291	13	max	54.838	2	-.078	15	.396	4	0	4	0	3	0	15
292		min	-49.128	9	-.325	4	-.024	3	0	10	0	5	-.001	4
293	14	max	54.771	2	-.12	15	.529	4	0	4	0	3	0	15
294		min	-49.184	9	-.503	4	-.024	3	0	10	0	5	-.001	4
295	15	max	54.704	2	-.162	15	.663	4	0	4	0	3	0	15
296		min	-49.24	9	-.681	4	-.024	3	0	10	0	10	0	4
297	16	max	54.637	2	-.204	15	.796	4	0	4	0	3	0	15
298		min	-49.296	9	-.859	4	-.024	3	0	10	0	10	0	4
299	17	max	54.57	2	-.245	15	.93	4	0	4	0	4	0	15
300		min	-49.352	9	-1.037	4	-.024	3	0	10	0	10	0	4
301	18	max	54.503	2	-.287	15	1.064	4	0	4	0	4	0	15
302		min	-49.408	9	-1.215	4	-.024	3	0	10	0	10	0	4
303	19	max	54.436	2	-.329	15	1.197	4	0	4	0	4	0	1
304		min	-49.464	9	-1.393	4	-.024	3	0	10	0	10	0	1
305	M12	1	max	347.927	1	0	.507	1	0	1	0	4	0	1
306		min	-118.873	3	0	1	-14.306	5	0	1	0	3	0	1
307	2	max	347.992	1	0	1	.507	1	0	1	0	1	0	1
308		min	-118.825	3	0	1	-14.362	5	0	1	-.001	5	0	1
309	3	max	348.056	1	0	1	.507	1	0	1	0	1	0	1
310		min	-118.776	3	0	1	-14.418	5	0	1	-.003	5	0	1
311	4	max	348.121	1	0	1	.507	1	0	1	0	1	0	1
312		min	-118.728	3	0	1	-14.475	5	0	1	-.004	5	0	1
313	5	max	348.186	1	0	1	.507	1	0	1	0	1	0	1



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Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-118.679	3	0	1	-14.531	5	0	1	-.005	5	0	1
315	6	max	348.25	1	0	1	.507	1	0	1	0	1	0	1
316		min	-118.631	3	0	1	-14.587	5	0	1	-.006	5	0	1
317	7	max	348.315	1	0	1	.507	1	0	1	0	1	0	1
318		min	-118.582	3	0	1	-14.643	5	0	1	-.008	5	0	1
319	8	max	348.38	1	0	1	.507	1	0	1	0	1	0	1
320		min	-118.534	3	0	1	-14.699	5	0	1	-.009	5	0	1
321	9	max	348.445	1	0	1	.507	1	0	1	0	1	0	1
322		min	-118.485	3	0	1	-14.755	5	0	1	-.01	5	0	1
323	10	max	348.509	1	0	1	.507	1	0	1	0	1	0	1
324		min	-118.436	3	0	1	-14.811	5	0	1	-.012	5	0	1
325	11	max	348.574	1	0	1	.507	1	0	1	0	1	0	1
326		min	-118.388	3	0	1	-14.867	5	0	1	-.013	5	0	1
327	12	max	348.639	1	0	1	.507	1	0	1	0	1	0	1
328		min	-118.339	3	0	1	-14.923	5	0	1	-.014	5	0	1
329	13	max	348.703	1	0	1	.507	1	0	1	0	1	0	1
330		min	-118.291	3	0	1	-14.979	5	0	1	-.016	5	0	1
331	14	max	348.768	1	0	1	.507	1	0	1	0	1	0	1
332		min	-118.242	3	0	1	-15.035	5	0	1	-.017	5	0	1
333	15	max	348.833	1	0	1	.507	1	0	1	0	1	0	1
334		min	-118.194	3	0	1	-15.091	5	0	1	-.018	5	0	1
335	16	max	348.897	1	0	1	.507	1	0	1	0	1	0	1
336		min	-118.145	3	0	1	-15.147	5	0	1	-.02	5	0	1
337	17	max	348.962	1	0	1	.507	1	0	1	0	1	0	1
338		min	-118.097	3	0	1	-15.204	5	0	1	-.021	5	0	1
339	18	max	349.027	1	0	1	.507	1	0	1	0	1	0	1
340		min	-118.048	3	0	1	-15.26	5	0	1	-.022	5	0	1
341	19	max	349.092	1	0	1	.507	1	0	1	0	1	0	1
342		min	-118	3	0	1	-15.316	5	0	1	-.024	5	0	1
343	M1	1	max	51.995	1	343.592	3	.383	10	0	.026	4	0	2
344		min	2.855	10	-254.011	1	-14.711	4	0	3	0	10	0	3
345	2	max	52.067	1	343.39	3	.383	10	0	1	.023	4	.055	1
346		min	2.915	10	-254.281	1	-14.469	4	0	3	0	10	-.075	3
347	3	max	61.408	1	4.49	4	.383	10	0	5	.019	4	.11	1
348		min	-5.875	3	-21.607	3	-13.387	4	0	1	0	10	-.148	3
349	4	max	61.48	1	4.144	14	.383	10	0	5	.017	4	.11	1
350		min	-5.821	3	-21.809	3	-13.145	4	0	1	0	10	-.143	3
351	5	max	61.553	1	3.879	14	.383	10	0	5	.014	4	.111	1
352		min	-5.766	3	-22.011	3	-12.903	4	0	1	0	10	-.138	3
353	6	max	61.625	1	3.614	14	.383	10	0	5	.011	4	.113	2
354		min	-5.712	3	-22.214	3	-12.661	4	0	1	0	10	-.134	3
355	7	max	61.697	1	3.349	14	.383	10	0	5	.008	4	.116	2
356		min	-5.658	3	-22.416	3	-12.419	4	0	1	0	10	-.129	3
357	8	max	61.769	1	3.084	14	.383	10	0	5	.006	4	.12	2
358		min	-5.604	3	-22.618	3	-12.177	4	0	1	0	10	-.124	3
359	9	max	61.842	1	2.819	14	.383	10	0	5	.003	4	.123	2
360		min	-5.55	3	-22.82	3	-12.166	1	0	1	0	10	-.119	3
361	10	max	61.914	1	2.554	14	.383	10	0	5	.001	3	.127	2
362		min	-5.495	3	-23.023	3	-12.166	1	0	1	0	10	-.114	3
363	11	max	61.986	1	2.289	14	.383	10	0	5	0	3	.131	2
364		min	-5.441	3	-23.225	3	-12.166	1	0	1	-.003	1	-.109	3
365	12	max	62.058	1	2.038	9	.383	10	0	5	0	10	.135	2
366		min	-5.387	3	-23.427	3	-12.166	1	0	1	-.005	1	-.104	3
367	13	max	62.131	1	1.813	9	.383	10	0	5	0	10	.138	2
368		min	-5.333	3	-23.63	3	-12.166	1	0	1	-.008	1	-.099	3
369	14	max	62.203	1	1.588	9	.383	10	0	5	0	10	.142	2
370		min	-5.279	3	-23.832	3	-12.166	1	0	1	-.01	1	-.094	3



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Dec 11, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	62.275	1	1.364	9	.383	10	0	5	0	10	.146	2
372			min	-5.224	3	-24.034	3	-12.166	1	0	1	-.013	1	-.088	3
373		16	max	69.225	2	16.466	2	.387	10	0	1	0	10	.15	2
374			min	-34.665	3	-50.69	3	-12.294	1	0	4	-.016	1	-.083	3
375		17	max	69.297	2	16.197	2	.387	10	0	1	0	10	.147	2
376			min	-34.611	3	-50.893	3	-12.294	1	0	4	-.019	1	-.072	3
377		18	max	-2.398	12	340.097	2	.409	10	0	3	0	10	.074	2
378			min	-52.016	1	-166.354	3	-23.731	4	0	2	-.021	1	-.036	3
379		19	max	-2.362	12	339.827	2	.409	10	0	3	0	10	0	2
380			min	-51.944	1	-166.556	3	-23.489	4	0	2	-.026	4	0	3
381	M5	1	max	129.039	1	1103.768	3	0	11	0	9	.03	4	0	3
382			min	-1.994	3	-811.527	1	-53.112	3	0	3	0	11	0	2
383		2	max	129.111	1	1103.566	3	0	11	0	9	.026	4	.175	1
384			min	-1.94	3	-811.797	1	-53.112	3	0	3	-.004	3	-.239	3
385		3	max	157.424	1	6.266	9	5.717	3	0	3	.021	4	.348	1
386			min	-42.944	3	-75.859	3	-16.504	4	0	4	-.015	3	-.473	3
387		4	max	157.496	1	6.041	9	5.717	3	0	3	.018	4	.353	1
388			min	-42.89	3	-76.061	3	-16.262	4	0	4	-.013	3	-.456	3
389		5	max	157.568	1	5.816	9	5.717	3	0	3	.014	4	.357	1
390			min	-42.836	3	-76.263	3	-16.02	4	0	4	-.012	3	-.44	3
391		6	max	157.64	1	5.591	9	5.717	3	0	3	.011	4	.362	1
392			min	-42.782	3	-76.466	3	-15.778	4	0	4	-.011	3	-.423	3
393		7	max	157.713	1	5.367	9	5.717	3	0	3	.007	4	.372	2
394			min	-42.727	3	-76.668	3	-15.536	4	0	4	-.01	3	-.407	3
395		8	max	157.785	1	5.142	9	5.717	3	0	3	.004	4	.384	2
396			min	-42.673	3	-76.87	3	-15.294	4	0	4	-.008	3	-.39	3
397		9	max	157.857	1	4.917	9	5.717	3	0	3	0	4	.396	2
398			min	-42.619	3	-77.072	3	-15.052	4	0	4	-.007	3	-.373	3
399		10	max	157.929	1	4.692	9	5.717	3	0	3	0	1	.408	2
400			min	-42.565	3	-77.275	3	-14.81	4	0	4	-.006	3	-.357	3
401		11	max	158.002	1	4.468	9	5.717	3	0	3	0	2	.42	2
402			min	-42.511	3	-77.477	3	-14.568	4	0	4	-.006	4	-.34	3
403		12	max	158.074	1	4.243	9	5.717	3	0	3	0	2	.432	2
404			min	-42.456	3	-77.679	3	-14.326	4	0	4	-.009	4	-.323	3
405		13	max	158.146	1	4.018	9	5.717	3	0	3	0	11	.445	2
406			min	-42.402	3	-77.882	3	-14.084	4	0	4	-.012	4	-.306	3
407		14	max	158.219	1	3.793	9	5.717	3	0	3	0	11	.457	2
408			min	-42.348	3	-78.084	3	-13.842	4	0	4	-.015	4	-.289	3
409		15	max	158.291	1	3.568	9	5.717	3	0	3	0	3	.469	2
410			min	-42.294	3	-78.286	3	-13.6	4	0	4	-.018	4	-.272	3
411		16	max	221.612	2	65.832	2	5.693	3	0	3	0	3	.481	2
412			min	-106.368	3	-134.086	3	-12.368	4	0	4	-.021	4	-.255	3
413		17	max	221.684	2	65.562	2	5.693	3	0	3	.002	3	.467	2
414			min	-106.314	3	-134.289	3	-12.126	4	0	4	-.024	4	-.226	3
415		18	max	-1.673	12	1086.514	2	5.261	3	0	4	.003	3	.235	2
416			min	-129.225	1	-524.935	3	-27.147	5	0	9	-.03	4	-.113	3
417		19	max	-1.636	12	1086.244	2	5.261	3	0	4	.004	3	0	3
418			min	-129.153	1	-525.138	3	-26.905	5	0	9	-.035	4	0	2
419	M9	1	max	51.961	1	343.539	3	112.2	4	0	3	0	5	0	2
420			min	-.596	5	-254.01	1	-.383	10	0	1	-.024	1	0	3
421		2	max	52.034	1	343.337	3	112.442	4	0	3	.025	5	.055	1
422			min	-.563	5	-254.28	1	-.383	10	0	1	-.021	1	-.075	3
423		3	max	61.704	1	4.049	9	12.061	1	0	4	.047	5	.109	1
424			min	-6.145	3	-21.52	3	-21.672	5	0	10	-.018	1	-.148	3
425		4	max	61.776	1	3.824	9	12.061	1	0	4	.042	5	.11	1
426			min	-6.091	3	-21.722	3	-21.43	5	0	10	-.016	1	-.143	3
427		5	max	61.848	1	3.599	9	12.061	1	0	4	.037	5	.111	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-6.036	3	-21.924	3	-21.188	5	0	10	-.013	1	-.138	3
429	6	max	61.92	1	3.375	9	12.061	1	0	4	.033	5	.113	2
430		min	-5.982	3	-22.126	3	-20.946	5	0	10	-.011	1	-.133	3
431	7	max	61.993	1	3.15	9	12.061	1	0	4	.028	5	.116	2
432		min	-5.928	3	-22.329	3	-20.704	5	0	10	-.008	1	-.129	3
433	8	max	62.065	1	2.925	9	12.061	1	0	4	.024	5	.12	2
434		min	-5.874	3	-22.531	3	-20.462	5	0	10	-.005	1	-.124	3
435	9	max	62.137	1	2.7	9	12.061	1	0	4	.019	5	.123	2
436		min	-5.82	3	-22.733	3	-20.22	5	0	10	-.003	1	-.119	3
437	10	max	62.209	1	2.476	9	12.061	1	0	4	.015	4	.127	2
438		min	-5.765	3	-22.936	3	-19.978	5	0	10	0	1	-.114	3
439	11	max	62.282	1	2.251	9	12.061	1	0	4	.011	4	.131	2
440		min	-5.711	3	-23.138	3	-19.736	5	0	10	0	10	-.109	3
441	12	max	62.354	1	2.026	9	12.061	1	0	4	.008	4	.135	2
442		min	-5.657	3	-23.34	3	-19.494	5	0	10	0	10	-.104	3
443	13	max	62.426	1	1.801	9	12.061	1	0	4	.008	1	.138	2
444		min	-5.603	3	-23.543	3	-19.252	5	0	10	0	10	-.099	3
445	14	max	62.498	1	1.577	9	12.061	1	0	4	.01	1	.142	2
446		min	-5.549	3	-23.745	3	-19.01	5	0	10	-.002	5	-.094	3
447	15	max	62.571	1	1.352	9	12.061	1	0	4	.013	1	.146	2
448		min	-5.494	3	-23.947	3	-18.768	5	0	10	-.006	5	-.088	3
449	16	max	69.291	2	16.221	2	12.197	1	0	10	.016	1	.15	2
450		min	-35.419	3	-51.041	3	-17.363	5	0	4	-.009	5	-.083	3
451	17	max	69.363	2	15.951	2	12.197	1	0	10	.018	1	.147	2
452		min	-35.365	3	-51.244	3	-17.121	5	0	4	-.013	5	-.072	3
453	18	max	9.177	5	340.097	2	12.694	1	0	2	.021	1	.074	2
454		min	-51.971	1	-166.348	3	-31.177	5	0	3	-.019	5	-.036	3
455	19	max	9.211	5	339.827	2	12.694	1	0	2	.024	1	0	2
456		min	-51.899	1	-166.55	3	-30.935	5	0	3	-.026	5	0	3
457	M13	1	max	112.199	4	253.858	1	.596	5	0	.024	1	0	1
458		min	-.383	10	-343.571	3	-51.959	1	0	3	0	5	0	3
459	2	max	107.834	4	180.64	1	1.205	5	0	2	.01	3	.131	3
460		min	-.383	10	-244.078	3	-39.015	1	0	3	-.002	10	-.097	1
461	3	max	103.469	4	107.423	1	1.813	5	0	2	.007	3	.217	3
462		min	-.383	10	-144.585	3	-26.071	1	0	3	-.011	1	-.161	1
463	4	max	99.104	4	34.205	1	2.421	5	0	2	.005	3	.259	3
464		min	-.383	10	-45.092	3	-13.127	1	0	3	-.019	1	-.192	1
465	5	max	94.739	4	54.402	3	3.03	5	0	2	.004	3	.257	3
466		min	-.383	10	-39.013	1	-3.548	3	0	3	-.022	1	-.191	1
467	6	max	90.374	4	153.895	3	12.762	1	0	2	.004	5	.211	3
468		min	-.383	10	-112.23	1	-2.975	3	0	3	-.02	1	-.157	1
469	7	max	86.008	4	253.388	3	25.706	1	0	2	.006	5	.12	3
470		min	-.383	10	-185.448	1	-2.403	3	0	3	-.011	1	-.091	1
471	8	max	81.643	4	352.881	3	38.65	1	0	2	.008	4	.007	1
472		min	-.383	10	-258.665	1	-1.831	3	0	3	0	12	-.014	3
473	9	max	77.278	4	452.374	3	51.595	1	0	2	.023	1	.139	1
474		min	-.383	10	-331.883	1	-1.259	3	0	3	0	3	-.193	3
475	10	max	72.913	4	12.624	5	64.539	1	0	2	.049	1	.302	1
476		min	-.383	10	-551.867	3	.606	12	0	3	-.017	5	-.417	3
477	11	max	49.616	4	331.883	1	7.325	5	0	3	.023	1	.139	1
478		min	-.383	10	-452.374	3	-51.561	1	0	2	-.014	5	-.193	3
479	12	max	45.251	4	258.665	1	7.933	5	0	3	.004	2	.007	1
480		min	-.383	10	-352.881	3	-38.617	1	0	2	-.01	5	-.014	3
481	13	max	40.885	4	185.448	1	8.542	5	0	3	0	10	.12	3
482		min	-.383	10	-253.388	3	-25.673	1	0	2	-.011	1	-.091	1
483	14	max	36.52	4	112.23	1	9.15	5	0	3	-.001	10	.211	3
484		min	-.383	10	-153.895	3	-12.729	1	0	2	-.02	1	-.157	1



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	32.155	4	39.013	1	10.139	4	0	3	.001	5	.257	3
486			min	-.383	10	-54.401	3	-2.116	2	0	2	-.022	1	-.191	1
487		16	max	27.79	4	45.092	3	13.267	4	0	3	.006	5	.259	3
488			min	-.383	10	-34.205	1	-.404	10	0	2	-.019	1	-.192	1
489		17	max	23.425	4	144.585	3	26.104	1	0	3	.011	5	.217	3
490			min	-.383	10	-107.423	1	.682	10	0	2	-.011	1	-.161	1
491		18	max	19.06	4	244.078	3	39.048	1	0	3	.017	4	.131	3
492			min	-.383	10	-180.64	1	1.768	10	0	2	-.002	10	-.097	1
493		19	max	14.694	4	343.571	3	51.992	1	0	3	.026	4	0	1
494			min	-.383	10	-253.858	1	2.855	10	0	2	0	10	0	3
495	M16	1	max	30.923	5	339.893	2	9.211	5	0	3	.024	1	0	2
496			min	-12.678	1	-166.564	3	-51.901	1	0	2	-.026	5	0	3
497		2	max	26.557	5	241.743	2	9.819	5	0	3	.004	1	.063	3
498			min	-12.678	1	-118.882	3	-38.957	1	0	2	-.022	5	-.129	2
499		3	max	22.192	5	143.593	2	10.428	5	0	3	0	12	.106	3
500			min	-12.678	1	-71.199	3	-26.013	1	0	2	-.019	4	-.215	2
501		4	max	17.827	5	45.443	2	11.036	5	0	3	-.001	12	.127	3
502			min	-12.678	1	-23.517	3	-13.069	1	0	2	-.019	1	-.257	2
503		5	max	13.462	5	24.166	3	11.644	5	0	3	-.002	12	.127	3
504			min	-12.678	1	-52.707	2	-2.231	3	0	2	-.022	1	-.255	2
505		6	max	9.097	5	71.848	3	14.545	4	0	3	-.001	10	.105	3
506			min	-12.678	1	-150.857	2	-1.659	3	0	2	-.019	1	-.21	2
507		7	max	4.731	5	119.531	3	25.764	1	0	3	.003	5	.063	3
508			min	-12.678	1	-249.007	2	-1.086	3	0	2	-.011	1	-.121	2
509		8	max	2.243	3	167.213	3	38.708	1	0	3	.01	4	.011	2
510			min	-12.678	1	-347.157	2	-.514	3	0	2	-.005	3	-.001	3
511		9	max	2.243	3	214.896	3	51.652	1	0	3	.024	1	.187	2
512			min	-12.678	1	-445.307	2	.058	3	0	2	-.005	3	-.086	3
513		10	max	18.546	5	-8.109	15	64.597	1	0	14	.049	1	.407	2
514			min	-12.678	1	-543.457	2	-1.481	3	0	2	-.004	3	-.192	3
515		11	max	14.181	5	445.307	2	6.393	5	0	2	.023	1	.187	2
516			min	-12.658	1	-214.896	3	-51.607	1	0	3	-.011	5	-.086	3
517		12	max	9.816	5	347.157	2	7.002	5	0	2	.004	2	.011	2
518			min	-12.658	1	-167.213	3	-38.663	1	0	3	-.008	5	-.001	3
519		13	max	5.451	5	249.007	2	7.61	5	0	2	0	10	.063	3
520			min	-12.658	1	-119.531	3	-25.719	1	0	3	-.011	1	-.121	2
521		14	max	1.086	5	150.857	2	8.218	5	0	2	0	12	.105	3
522			min	-12.658	1	-71.848	3	-12.774	1	0	3	-.019	1	-.21	2
523		15	max	.409	10	52.707	2	9.183	4	0	2	.003	5	.127	3
524			min	-12.658	1	-24.166	3	-2.157	2	0	3	-.022	1	-.255	2
525		16	max	.409	10	23.517	3	13.114	1	0	2	.007	5	.127	3
526			min	-12.658	1	-45.443	2	-.425	10	0	3	-.019	1	-.257	2
527		17	max	.409	10	71.199	3	26.058	1	0	2	.011	5	.106	3
528			min	-14.776	4	-143.593	2	.661	10	0	3	-.011	1	-.215	2
529		18	max	.409	10	118.882	3	39.002	1	0	2	.017	4	.063	3
530			min	-19.141	4	-241.743	2	1.748	10	0	3	-.002	10	-.129	2
531		19	max	.409	10	166.564	3	51.947	1	0	2	.026	4	0	2
532			min	-23.506	4	-339.893	2	2.361	12	0	3	0	10	0	3
533	M15	1	max	0	1	.824	3	.136	3	0	1	0	1	0	1
534			min	-69.458	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.732	3	.136	3	0	1	0	1	0	1
536			min	-69.512	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.641	3	.136	3	0	1	0	1	0	1
538			min	-69.566	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.549	3	.136	3	0	1	0	1	0	1
540			min	-69.62	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.458	3	.136	3	0	1	0	1	0	1



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Dec 11, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-69.674	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.366	3	.136	3	0	1	0	1	0	1
544			min	-69.728	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.275	3	.136	3	0	1	0	3	0	1
546			min	-69.782	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.183	3	.136	3	0	1	0	3	0	1
548			min	-69.836	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.092	3	.136	3	0	1	0	3	0	1
550			min	-69.89	3	0	1	0	1	0	3	0	1	0	3
551		10	max	0	1	0	1	.136	3	0	1	0	3	0	1
552			min	-69.944	3	0	1	0	1	0	3	0	1	0	3
553		11	max	0	1	0	1	.136	3	0	1	0	3	0	1
554			min	-69.998	3	-.092	3	0	1	0	3	0	1	0	3
555		12	max	0	1	0	1	.136	3	0	1	0	3	0	1
556			min	-70.052	3	-.183	3	0	1	0	3	0	1	0	3
557		13	max	0	1	0	1	.136	3	0	1	0	3	0	1
558			min	-70.106	3	-.275	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.136	3	0	1	0	3	0	1
560			min	-70.16	3	-.366	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.136	3	0	1	0	3	0	1
562			min	-70.214	3	-.458	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.136	3	0	1	0	3	0	1
564			min	-70.268	3	-.549	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.136	3	0	1	0	3	0	1
566			min	-70.322	3	-.641	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.136	3	0	1	0	3	0	1
568			min	-70.376	3	-.732	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.136	3	0	1	0	3	0	1
570			min	-70.43	3	-.824	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	1.961	4	.239	4	0	3	0	3	0	1
572			min	-157.988	4	0	2	-.053	3	0	1	0	4	0	1
573		2	max	0	2	1.743	4	.217	4	0	3	0	3	0	2
574			min	-158	4	0	2	-.053	3	0	1	0	4	0	4
575		3	max	0	2	1.525	4	.195	4	0	3	0	3	0	2
576			min	-158.011	4	0	2	-.053	3	0	1	0	4	0	4
577		4	max	0	2	1.307	4	.173	4	0	3	0	3	0	2
578			min	-158.023	4	0	2	-.053	3	0	1	0	1	-.001	4
579		5	max	0	2	1.09	4	.151	4	0	3	0	3	0	2
580			min	-158.035	4	0	2	-.053	3	0	1	0	1	-.002	4
581		6	max	0	2	.872	4	.128	4	0	3	0	3	0	2
582			min	-158.046	4	0	2	-.053	3	0	1	0	1	-.002	4
583		7	max	0	2	.654	4	.106	4	0	3	0	3	0	2
584			min	-158.058	4	0	2	-.053	3	0	1	0	1	-.002	4
585		8	max	0	2	.436	4	.084	4	0	3	0	5	0	2
586			min	-158.07	4	0	2	-.053	3	0	1	0	1	-.002	4
587		9	max	0	2	.218	4	.062	4	0	3	0	5	0	2
588			min	-158.081	4	0	2	-.053	3	0	1	0	1	-.002	4
589		10	max	0	2	0	1	.046	1	0	3	0	5	0	2
590			min	-158.093	4	0	1	-.053	3	0	1	0	1	-.002	4
591		11	max	0	2	0	2	.046	1	0	3	0	5	0	2
592			min	-158.105	4	-.218	4	-.053	3	0	1	0	1	-.002	4
593		12	max	0	2	0	2	.046	1	0	3	0	5	0	2
594			min	-158.116	4	-.436	4	-.053	3	0	1	0	1	-.002	4
595		13	max	.007	1	0	2	.046	1	0	3	0	5	0	2
596			min	-158.128	4	-.654	4	-.053	3	0	1	0	9	-.002	4
597		14	max	.079	1	0	2	.046	1	0	3	0	5	0	2
598			min	-158.14	4	-.872	4	-.053	5	0	1	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.151	1	0	2	.046	1	0	3	0	4	0	2
600		min	-158.151	4	-1.09	4	-.075	5	0	1	0	3	-.002	4
601	16	max	.223	1	0	2	.046	1	0	3	0	4	0	2
602		min	-158.163	4	-1.307	4	-.098	5	0	1	0	3	-.001	4
603	17	max	.295	1	0	2	.046	1	0	3	0	1	0	2
604		min	-158.175	4	-1.525	4	-.12	5	0	1	0	3	0	4
605	18	max	.367	1	0	2	.046	1	0	3	0	1	0	2
606		min	-158.204	5	-1.743	4	-.142	5	0	1	0	3	0	4
607	19	max	.439	1	0	2	.046	1	0	3	0	1	0	1
608		min	-158.275	5	-1.961	4	-.164	5	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.005	2	.002	1	8.691e-4	5	NC	3	NC	1
2			min	-.003	3	-.005	3	-.008	5	-1.795e-4	1	5552.937	2	NC	1
3		2	max	.002	1	.005	2	.002	1	8.879e-4	5	NC	3	NC	1
4			min	-.003	3	-.004	3	-.008	5	-1.718e-4	1	6034.955	2	NC	1
5		3	max	.002	1	.005	2	.002	1	9.067e-4	5	NC	1	NC	1
6			min	-.002	3	-.004	3	-.008	5	-1.642e-4	1	6604.045	2	NC	1
7		4	max	.002	1	.004	2	.001	1	9.256e-4	5	NC	1	NC	1
8			min	-.002	3	-.004	3	-.007	5	-1.565e-4	1	7280.82	2	NC	1
9		5	max	.001	1	.004	2	.001	1	9.444e-4	5	NC	1	NC	1
10			min	-.002	3	-.004	3	-.007	5	-1.488e-4	1	8092.643	2	NC	1
11	6	max	.001	1	.003	2	.001	1	9.633e-4	5	NC	1	NC	1	
12		min	-.002	3	-.004	3	-.007	5	-1.411e-4	1	9076.45	2	NC	1	
13	7	max	.001	1	.003	2	0	1	9.821e-4	5	NC	1	NC	1	
14		min	-.002	3	-.003	3	-.006	5	-1.334e-4	1	NC	1	NC	1	
15	8	max	.001	1	.003	2	0	1	1.001e-3	5	NC	1	NC	1	
16		min	-.002	3	-.003	3	-.006	5	-1.257e-4	1	NC	1	NC	1	
17	9	max	.001	1	.002	2	0	1	1.02e-3	5	NC	1	NC	1	
18		min	-.002	3	-.003	3	-.005	5	-1.18e-4	1	NC	1	NC	1	
19	10	max	0	1	.002	2	0	1	1.039e-3	5	NC	1	NC	1	
20		min	-.001	3	-.003	3	-.005	5	-1.104e-4	1	NC	1	NC	1	
21	11	max	0	1	.002	2	0	1	1.057e-3	5	NC	1	NC	1	
22		min	-.001	3	-.003	3	-.005	5	-1.027e-4	1	NC	1	NC	1	
23	12	max	0	1	.001	2	0	1	1.076e-3	5	NC	1	NC	1	
24		min	-.001	3	-.002	3	-.004	5	-9.499e-5	1	NC	1	NC	1	
25	13	max	0	1	.001	2	0	1	1.095e-3	5	NC	1	NC	1	
26		min	0	3	-.002	3	-.004	5	-8.73e-5	1	NC	1	NC	1	
27	14	max	0	1	0	2	0	1	1.114e-3	5	NC	1	NC	1	
28		min	0	3	-.002	3	-.003	5	-7.962e-5	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	1	1.133e-3	5	NC	1	NC	1	
30		min	0	3	-.001	3	-.002	5	-7.193e-5	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.152e-3	5	NC	1	NC	1	
32		min	0	3	-.001	3	-.002	5	-6.425e-5	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.171e-3	5	NC	1	NC	1	
34		min	0	3	0	3	-.001	5	-5.656e-5	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.189e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-4.888e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.208e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-4.119e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.879e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.5e-4	5	NC	1	NC	1
41		2	max	0	14	0	2	.003	5	2.611e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-5.535e-4	5	NC	1	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	14	0	2	.006	5	3.342e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-5.57e-4	5	NC	1	NC	1
45		4	max	0	14	0	2	.009	4	4.073e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-5.606e-4	5	NC	1	NC	1
47		5	max	0	14	0	2	.012	4	4.805e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.641e-4	5	NC	1	NC	1
49		6	max	0	14	0	2	.015	4	5.536e-5	1	NC	1	NC	1
50			min	0	2	-.003	3	0	9	-5.676e-4	5	NC	1	NC	1
51		7	max	0	14	0	2	.018	4	6.268e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-5.711e-4	5	NC	1	NC	1
53		8	max	0	14	0	2	.02	4	6.999e-5	1	NC	1	NC	1
54			min	0	2	-.004	3	0	9	-5.747e-4	5	NC	1	NC	1
55		9	max	0	14	0	2	.023	4	7.73e-5	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-5.782e-4	5	NC	1	NC	1
57		10	max	0	14	.001	2	.026	4	8.462e-5	1	NC	1	NC	1
58			min	0	2	-.005	3	0	10	-5.817e-4	5	NC	1	NC	1
59		11	max	0	14	.002	2	.028	4	9.193e-5	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-5.853e-4	5	NC	1	NC	1
61		12	max	0	14	.002	2	.031	4	9.925e-5	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-5.888e-4	5	NC	1	NC	1
63		13	max	0	14	.003	2	.033	4	1.066e-4	1	NC	1	NC	1
64			min	0	2	-.006	3	0	10	-5.923e-4	5	NC	1	NC	1
65		14	max	0	14	.003	2	.036	4	1.139e-4	1	NC	1	NC	1
66			min	0	2	-.006	3	0	10	-5.959e-4	5	NC	1	NC	1
67		15	max	0	14	.004	2	.038	4	1.212e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-5.994e-4	5	NC	1	NC	1
69		16	max	0	14	.005	2	.04	4	1.285e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-6.029e-4	5	9142.315	2	NC	1
71		17	max	0	14	.006	2	.042	4	1.358e-4	1	NC	1	NC	1
72			min	0	2	-.007	3	0	10	-6.065e-4	5	7770.866	2	NC	1
73		18	max	0	14	.007	2	.044	4	1.431e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	10	-6.1e-4	5	6717.76	2	NC	1
75		19	max	0	14	.008	2	.047	4	1.504e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	10	-6.135e-4	5	5899.813	2	NC	1
77	M4	1	max	.002	1	.006	2	0	10	1.979e-3	5	NC	1	NC	1
78			min	0	3	-.005	3	-.049	4	-1.538e-4	1	NC	1	392.499	4
79		2	max	.002	1	.006	2	0	10	1.979e-3	5	NC	1	NC	1
80			min	0	3	-.005	3	-.045	4	-1.538e-4	1	NC	1	427.828	4
81		3	max	.001	1	.006	2	0	10	1.979e-3	5	NC	1	NC	1
82			min	0	3	-.004	3	-.041	4	-1.538e-4	1	NC	1	469.87	4
83		4	max	.001	1	.005	2	0	10	1.979e-3	5	NC	1	NC	1
84			min	0	3	-.004	3	-.037	4	-1.538e-4	1	NC	1	520.394	4
85		5	max	.001	1	.005	2	0	10	1.979e-3	5	NC	1	NC	1
86			min	0	3	-.004	3	-.033	4	-1.538e-4	1	NC	1	581.807	4
87		6	max	.001	1	.004	2	0	10	1.979e-3	5	NC	1	NC	1
88			min	0	3	-.004	3	-.029	4	-1.538e-4	1	NC	1	657.459	4
89		7	max	.001	1	.004	2	0	10	1.979e-3	5	NC	1	NC	1
90			min	0	3	-.003	3	-.026	4	-1.538e-4	1	NC	1	752.12	4
91		8	max	.001	1	.004	2	0	10	1.979e-3	5	NC	1	NC	1
92			min	0	3	-.003	3	-.022	4	-1.538e-4	1	NC	1	872.771	4
93		9	max	0	1	.003	2	0	10	1.979e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.019	4	-1.538e-4	1	NC	1	1029.975	4
95		10	max	0	1	.003	2	0	10	1.979e-3	5	NC	1	NC	1
96			min	0	3	-.002	3	-.016	4	-1.538e-4	1	NC	1	1240.334	4
97		11	max	0	1	.003	2	0	10	1.979e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.013	4	-1.538e-4	1	NC	1	1531.187	4
99		12	max	0	1	.002	2	0	10	1.979e-3	5	NC	1	NC	1







Company : Schletter, Inc.
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Job Number :
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Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.006	3	-.01	4	-1.845e-4	3	NC	1	1906.749	4
215		13	max	.002	2	.007	2	0	9	1.81e-3	4	NC	1	NC	1
216			min	0	3	-.005	3	-.008	4	-1.845e-4	3	NC	1	2529.243	4
217		14	max	.001	2	.006	2	0	9	1.81e-3	4	NC	1	NC	1
218			min	0	3	-.004	3	-.005	4	-1.845e-4	3	NC	1	3544.903	4
219		15	max	.001	2	.004	2	0	9	1.81e-3	4	NC	1	NC	1
220			min	0	3	-.003	3	-.004	4	-1.845e-4	3	NC	1	5377.482	4
221		16	max	0	2	.003	2	0	9	1.81e-3	4	NC	1	NC	1
222			min	0	3	-.002	3	-.002	4	-1.845e-4	3	NC	1	9231.542	4
223		17	max	0	2	.002	2	0	9	1.81e-3	4	NC	1	NC	1
224			min	0	3	-.002	3	0	4	-1.845e-4	3	NC	1	NC	1
225		18	max	0	2	.001	2	0	9	1.81e-3	4	NC	1	NC	1
226			min	0	3	0	3	0	4	-1.845e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	1.81e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.845e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.005	2	0	3	1.835e-4	1	NC	3	NC	1
230			min	-.002	3	-.005	3	-.003	4	-3.959e-4	3	5563.113	2	NC	1
231		2	max	.002	1	.005	2	0	3	1.945e-4	4	NC	3	NC	1
232			min	-.002	3	-.004	3	-.003	4	-3.848e-4	3	6046.237	2	NC	1
233		3	max	.002	1	.005	2	0	3	2.376e-4	4	NC	1	NC	1
234			min	-.002	3	-.004	3	-.003	4	-3.736e-4	3	6616.675	2	NC	1
235		4	max	.002	1	.004	2	0	3	2.807e-4	4	NC	1	NC	1
236			min	-.002	3	-.004	3	-.003	4	-3.624e-4	3	7295.105	2	NC	1
237		5	max	.001	1	.004	2	0	3	3.239e-4	4	NC	1	NC	1
238			min	-.002	3	-.004	3	-.003	4	-3.513e-4	3	8108.981	2	NC	1
239		6	max	.001	1	.003	2	0	3	3.67e-4	4	NC	1	NC	1
240			min	-.002	3	-.004	3	-.003	4	-3.401e-4	3	9095.365	2	NC	1
241		7	max	.001	1	.003	2	0	3	4.101e-4	4	NC	1	NC	1
242			min	-.002	3	-.004	3	-.003	4	-3.29e-4	3	NC	1	NC	1
243		8	max	.001	1	.003	2	0	3	4.532e-4	4	NC	1	NC	1
244			min	-.001	3	-.003	3	-.003	4	-3.178e-4	3	NC	1	NC	1
245		9	max	.001	1	.002	2	0	3	4.964e-4	4	NC	1	NC	1
246			min	-.001	3	-.003	3	-.003	4	-3.067e-4	3	NC	1	NC	1
247		10	max	0	1	.002	2	0	3	5.395e-4	4	NC	1	NC	1
248			min	-.001	3	-.003	3	-.003	4	-2.955e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	5.826e-4	4	NC	1	NC	1
250			min	-.001	3	-.003	3	-.003	4	-2.844e-4	3	NC	1	NC	1
251		12	max	0	1	.001	2	0	3	6.257e-4	4	NC	1	NC	1
252			min	0	3	-.002	3	-.003	4	-2.732e-4	3	NC	1	NC	1
253		13	max	0	1	0	2	0	3	6.688e-4	4	NC	1	NC	1
254			min	0	3	-.002	3	-.002	4	-2.621e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	7.12e-4	4	NC	1	NC	1
256			min	0	3	-.002	3	-.002	4	-2.509e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	7.551e-4	4	NC	1	NC	1
258			min	0	3	-.002	3	-.002	4	-2.398e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	7.982e-4	4	NC	1	NC	1
260			min	0	3	-.001	3	-.001	4	-2.286e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	8.413e-4	4	NC	1	NC	1
262			min	0	3	0	3	0	4	-2.175e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	8.845e-4	4	NC	1	NC	1
264			min	0	3	0	3	0	4	-2.063e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	9.276e-4	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.952e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.909e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-4.227e-4	4	NC	1	NC	1
269		2	max	0	9	0	2	.002	4	7.188e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-4.65e-4	4	NC	1	NC	1



Company : Schletter, Inc.
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Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	9	0	2	.005	4	5.466e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-5.073e-4	4	NC	1	NC	1
273		4	max	0	9	0	2	.007	4	3.745e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.496e-4	4	NC	1	NC	1
275		5	max	0	9	0	2	.009	4	2.024e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.919e-4	4	NC	1	NC	1
277		6	max	0	9	0	2	.012	4	3.028e-6	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-6.342e-4	4	NC	1	NC	1
279		7	max	0	9	0	2	.014	4	2.09e-6	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-6.765e-4	4	NC	1	NC	1
281		8	max	0	9	0	2	.017	4	2.385e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-7.188e-4	4	NC	1	NC	1
283		9	max	0	9	0	2	.019	5	2.68e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	3	-7.611e-4	4	NC	1	NC	1
285		10	max	0	9	.001	2	.021	5	2.974e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-8.034e-4	4	NC	1	NC	1
287		11	max	0	9	.002	2	.024	5	3.269e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	3	-8.457e-4	4	NC	1	NC	1
289		12	max	0	9	.002	2	.026	5	3.564e-6	10	NC	1	NC	1
290			min	0	2	-.006	3	-.003	3	-8.881e-4	4	NC	1	NC	1
291		13	max	0	9	.003	2	.028	5	3.859e-6	10	NC	1	NC	1
292			min	0	2	-.006	3	-.003	3	-9.304e-4	4	NC	1	NC	1
293		14	max	0	9	.003	2	.031	5	4.154e-6	10	NC	1	NC	1
294			min	0	2	-.007	3	-.003	3	-9.727e-4	4	NC	1	NC	1
295		15	max	0	9	.004	2	.033	5	4.448e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.002	3	-1.015e-3	4	NC	1	NC	1
297		16	max	0	9	.005	2	.035	5	4.743e-6	10	NC	1	NC	1
298			min	0	2	-.007	3	-.002	3	-1.057e-3	4	9153.918	2	NC	1
299		17	max	0	9	.006	2	.038	5	5.038e-6	10	NC	1	NC	1
300			min	0	2	-.007	3	-.002	3	-1.1e-3	4	7779.607	2	NC	1
301		18	max	0	9	.007	2	.04	5	5.333e-6	10	NC	3	NC	1
302			min	0	2	-.007	3	-.002	3	-1.142e-3	4	6724.555	2	NC	1
303		19	max	0	9	.008	2	.042	5	5.628e-6	10	NC	3	NC	1
304			min	0	2	-.007	3	-.002	1	-1.184e-3	4	5905.256	2	NC	1
305	M12	1	max	.002	1	.006	2	.002	1	2.454e-3	4	NC	1	NC	1
306			min	0	3	-.005	3	-.046	5	-5.621e-6	10	NC	1	417.079	5
307		2	max	.002	1	.006	2	.001	1	2.454e-3	4	NC	1	NC	1
308			min	0	3	-.005	3	-.043	5	-5.621e-6	10	NC	1	454.611	5
309		3	max	.001	1	.006	2	.001	1	2.454e-3	4	NC	1	NC	1
310			min	0	3	-.004	3	-.039	5	-5.621e-6	10	NC	1	499.274	5
311		4	max	.001	1	.005	2	.001	1	2.454e-3	4	NC	1	NC	1
312			min	0	3	-.004	3	-.035	5	-5.621e-6	10	NC	1	552.946	5
313		5	max	.001	1	.005	2	.001	1	2.454e-3	4	NC	1	NC	1
314			min	0	3	-.004	3	-.031	5	-5.621e-6	10	NC	1	618.185	5
315		6	max	.001	1	.004	2	0	1	2.454e-3	4	NC	1	NC	1
316			min	0	3	-.004	3	-.028	5	-5.621e-6	10	NC	1	698.548	5
317		7	max	.001	1	.004	2	0	1	2.454e-3	4	NC	1	NC	1
318			min	0	3	-.003	3	-.024	5	-5.621e-6	10	NC	1	799.101	5
319		8	max	.001	1	.004	2	0	1	2.454e-3	4	NC	1	NC	1
320			min	0	3	-.003	3	-.021	5	-5.621e-6	10	NC	1	927.262	5
321		9	max	0	1	.003	2	0	1	2.454e-3	4	NC	1	NC	1
322			min	0	3	-.003	3	-.018	5	-5.621e-6	10	NC	1	1094.246	5
323		10	max	0	1	.003	2	0	1	2.454e-3	4	NC	1	NC	1
324			min	0	3	-.002	3	-.015	5	-5.621e-6	10	NC	1	1317.689	5
325		11	max	0	1	.003	2	0	1	2.454e-3	4	NC	1	NC	1
326			min	0	3	-.002	3	-.012	5	-5.621e-6	10	NC	1	1626.627	5
327		12	max	0	1	.002	2	0	1	2.454e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Dec 11, 2015

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.002	3	-.009	5	-5.621e-6	10	NC	1	2071.71	5
329		max	0	1	.002	2	0	1	2.454e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.007	5	-5.621e-6	10	NC	1	2747.933	5
331		max	0	1	.002	2	0	1	2.454e-3	4	NC	1	NC	1
332		min	0	3	-.001	3	-.005	5	-5.621e-6	10	NC	1	3851.232	5
333		max	0	1	.001	2	0	1	2.454e-3	4	NC	1	NC	1
334		min	0	3	-.001	3	-.003	5	-5.621e-6	10	NC	1	5841.892	5
335		max	0	1	.001	2	0	1	2.454e-3	4	NC	1	NC	1
336		min	0	3	0	3	-.002	5	-5.621e-6	10	NC	1	NC	1
337		max	0	1	0	2	0	1	2.454e-3	4	NC	1	NC	1
338		min	0	3	0	3	0	5	-5.621e-6	10	NC	1	NC	1
339		max	0	1	0	2	0	1	2.454e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	-5.621e-6	10	NC	1	NC	1
341		max	0	1	0	1	0	1	2.454e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	-5.621e-6	10	NC	1	NC	1
343	M1	max	.005	3	.021	3	.004	5	6.458e-3	1	NC	1	NC	1
344		min	-.005	2	-.018	2	0	9	-8.51e-3	3	NC	1	NC	1
345		max	.005	3	.011	3	.006	5	3.161e-3	1	NC	4	NC	1
346		min	-.005	2	-.01	1	-.001	1	-4.179e-3	3	5008.146	3	NC	1
347		max	.005	3	.002	3	.008	5	1.654e-4	5	NC	4	NC	1
348		min	-.005	2	-.002	1	-.002	1	-7.585e-5	1	2600.548	3	NC	1
349		max	.005	3	.006	2	.011	5	1.576e-4	5	NC	4	NC	1
350		min	-.005	2	-.005	3	-.002	1	-5.902e-5	1	1862.214	3	7268.083	5
351		max	.005	3	.012	2	.014	5	1.498e-4	5	NC	4	NC	1
352		min	-.005	2	-.011	3	-.002	1	-4.278e-5	9	1511.964	3	5177.186	5
353		max	.005	3	.017	2	.016	5	1.421e-4	5	NC	4	NC	1
354		min	-.005	2	-.016	3	-.002	1	-3.011e-5	9	1292.473	2	3963.78	5
355		max	.005	3	.021	2	.019	5	1.343e-4	5	NC	4	NC	1
356		min	-.006	2	-.02	3	-.002	1	-1.745e-5	9	1154.862	2	3182.179	5
357		max	.005	3	.024	2	.022	5	1.265e-4	5	NC	5	NC	1
358		min	-.006	2	-.022	3	-.001	1	-4.778e-6	9	1069.143	2	2643.06	5
359		max	.005	3	.026	2	.025	5	1.223e-4	4	NC	5	NC	1
360		min	-.006	2	-.023	3	0	1	-9.682e-7	10	1019.076	2	2249.169	4
361		max	.005	3	.026	2	.028	4	1.185e-4	4	NC	5	NC	1
362		min	-.006	2	-.024	3	0	9	-1.546e-6	10	996.96	2	1945.74	4
363		max	.005	3	.026	2	.032	4	1.147e-4	4	NC	5	NC	1
364		min	-.006	2	-.023	3	0	9	-2.124e-6	10	1000.092	2	1714.048	4
365		max	.005	3	.024	2	.035	4	1.108e-4	4	NC	4	NC	1
366		min	-.006	2	-.021	3	0	10	-2.702e-6	10	1029.738	2	1533.282	4
367		max	.005	3	.022	2	.038	4	1.07e-4	4	NC	4	NC	1
368		min	-.006	2	-.018	3	0	10	-3.28e-6	10	1091.774	2	1389.895	4
369		max	.005	3	.017	2	.041	4	1.092e-4	1	NC	4	NC	1
370		min	-.006	2	-.014	3	0	10	-3.858e-6	10	1199.606	2	1274.742	4
371		max	.005	3	.012	2	.044	4	1.26e-4	1	NC	4	NC	1
372		min	-.006	2	-.01	3	0	10	-4.435e-6	10	1382.456	2	1181.471	4
373		max	.005	3	.005	2	.047	4	2.502e-4	4	NC	4	NC	1
374		min	-.006	2	-.004	3	0	10	-4.867e-6	10	1711.078	2	1105.559	4
375		max	.005	3	.002	3	.049	4	4.081e-3	4	NC	4	NC	1
376		min	-.006	2	-.003	2	0	10	-1.822e-6	10	2405.642	2	1043.786	4
377		max	.005	3	.009	3	.051	4	4.258e-3	2	NC	4	NC	1
378		min	-.006	2	-.013	2	0	10	-2.179e-3	3	4647.749	2	993.575	4
379		max	.005	3	.016	3	.053	4	8.58e-3	2	NC	1	NC	1
380		min	-.006	2	-.023	2	0	9	-4.441e-3	3	NC	1	954.176	4
381	M5	max	.014	3	.066	3	.004	5	9.256e-6	4	NC	1	NC	1
382		min	-.018	2	-.058	1	0	9	0	1	NC	1	NC	1
383		max	.014	3	.036	3	.006	5	7.737e-5	5	NC	4	NC	1
384		min	-.018	2	-.03	1	0	9	-1.152e-5	9	1584.62	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.014	3	.008	3	.008	5	1.487e-4	3	NC	5	NC	1
386		min	-.018	2	-.005	1	0	9	-2.285e-5	9	823.15	3	NC	1
387	4	max	.014	3	.018	2	.011	5	1.494e-4	5	NC	5	NC	1
388		min	-.018	2	-.016	3	0	9	-2.141e-5	9	590.018	3	NC	1
389	5	max	.014	3	.038	2	.014	5	1.544e-4	5	NC	5	NC	1
390		min	-.018	2	-.035	3	0	9	-1.998e-5	9	473.919	2	NC	1
391	6	max	.014	3	.054	2	.017	5	1.594e-4	5	NC	5	NC	1
392		min	-.018	2	-.05	3	0	9	-1.855e-5	9	403.759	2	NC	1
393	7	max	.014	3	.067	2	.02	5	1.643e-4	5	NC	5	NC	1
394		min	-.018	2	-.062	3	0	9	-1.712e-5	9	360.644	2	NC	1
395	8	max	.014	3	.077	2	.023	4	1.693e-4	5	NC	5	NC	1
396		min	-.018	2	-.069	3	0	9	-1.568e-5	9	333.776	2	NC	1
397	9	max	.014	3	.083	2	.027	4	1.743e-4	5	NC	5	NC	1
398		min	-.018	2	-.073	3	0	9	-1.425e-5	9	318.07	2	NC	1
399	10	max	.014	3	.085	2	.03	4	1.793e-4	5	NC	5	NC	1
400		min	-.018	2	-.074	3	0	9	-1.282e-5	9	311.112	2	NC	1
401	11	max	.014	3	.084	2	.034	4	1.843e-4	5	NC	5	NC	1
402		min	-.018	2	-.071	3	0	9	-1.139e-5	9	312.051	2	NC	1
403	12	max	.014	3	.078	2	.037	4	1.893e-4	5	NC	5	NC	1
404		min	-.018	2	-.065	3	0	9	-9.956e-6	9	321.281	2	NC	1
405	13	max	.014	3	.069	2	.04	4	1.944e-4	4	NC	5	NC	1
406		min	-.018	2	-.056	3	0	9	-8.524e-6	9	340.636	2	NC	1
407	14	max	.014	3	.056	2	.043	4	2.001e-4	4	NC	5	NC	1
408		min	-.018	2	-.045	3	0	9	-7.092e-6	9	374.302	2	NC	1
409	15	max	.014	3	.038	2	.046	4	2.058e-4	4	NC	5	NC	1
410		min	-.018	2	-.031	3	0	9	-5.66e-6	9	431.411	2	NC	1
411	16	max	.014	3	.017	2	.048	4	3.63e-4	4	NC	5	NC	1
412		min	-.018	2	-.014	3	0	9	-4.954e-6	9	534.081	2	NC	1
413	17	max	.014	3	.005	3	.05	4	4.128e-3	4	NC	5	NC	1
414		min	-.018	2	-.01	2	0	9	-2.153e-5	9	751.246	2	NC	1
415	18	max	.014	3	.027	3	.052	4	2.12e-3	4	NC	4	NC	1
416		min	-.018	2	-.041	2	0	9	-1.105e-5	9	1451.896	2	NC	1
417	19	max	.014	3	.049	3	.053	4	4.11e-6	5	NC	1	NC	1
418		min	-.018	2	-.074	2	0	9	-4.699e-7	3	NC	1	NC	1
419	M9	1	max	.005	.021	3	.004	5	8.517e-3	3	NC	1	NC	1
420		min	-.005	2	-.018	2	0	9	-6.458e-3	1	NC	1	NC	1
421	2	max	.005	3	.011	3	.003	4	4.236e-3	3	NC	4	NC	1
422		min	-.005	2	-.01	1	0	10	-3.178e-3	1	5010.706	3	NC	1
423	3	max	.005	3	.002	3	.003	4	4.189e-5	1	NC	4	NC	1
424		min	-.005	2	-.002	1	0	3	-3.487e-5	5	2601.913	3	NC	1
425	4	max	.005	3	.006	2	.004	4	2.668e-5	1	NC	4	NC	1
426		min	-.005	2	-.005	3	-.001	3	-5.05e-5	5	1863.19	3	NC	1
427	5	max	.005	3	.012	2	.005	4	1.366e-5	11	NC	4	NC	1
428		min	-.005	2	-.011	3	-.002	3	-6.677e-5	4	1512.722	3	NC	1
429	6	max	.005	3	.017	2	.007	4	6.152e-6	11	NC	4	NC	1
430		min	-.005	2	-.016	3	-.003	3	-8.541e-5	4	1292.72	2	NC	1
431	7	max	.005	3	.021	2	.01	4	-4.461e-8	10	NC	4	NC	1
432		min	-.005	2	-.02	3	-.003	3	-1.041e-4	4	1155.093	2	7726.235	4
433	8	max	.005	3	.024	2	.012	4	5.25e-7	10	NC	5	NC	1
434		min	-.006	2	-.022	3	-.003	3	-1.227e-4	4	1069.366	2	5309.222	4
435	9	max	.005	3	.026	2	.016	4	1.095e-6	10	NC	5	NC	1
436		min	-.006	2	-.024	3	-.004	3	-1.413e-4	4	1019.297	2	3924.807	4
437	10	max	.005	3	.026	2	.019	4	1.664e-6	10	NC	5	NC	1
438		min	-.006	2	-.024	3	-.004	3	-1.6e-4	4	997.184	2	3053.724	4
439	11	max	.005	3	.026	2	.023	5	2.234e-6	10	NC	5	NC	1
440		min	-.006	2	-.023	3	-.004	3	-1.786e-4	4	1000.323	2	2468.329	4
441	12	max	.005	3	.024	2	.027	5	2.803e-6	10	NC	5	NC	1





Anchor Designer™
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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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