



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.07	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	120 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.741 k-ft
$M_z$ =	0.402 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>97%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

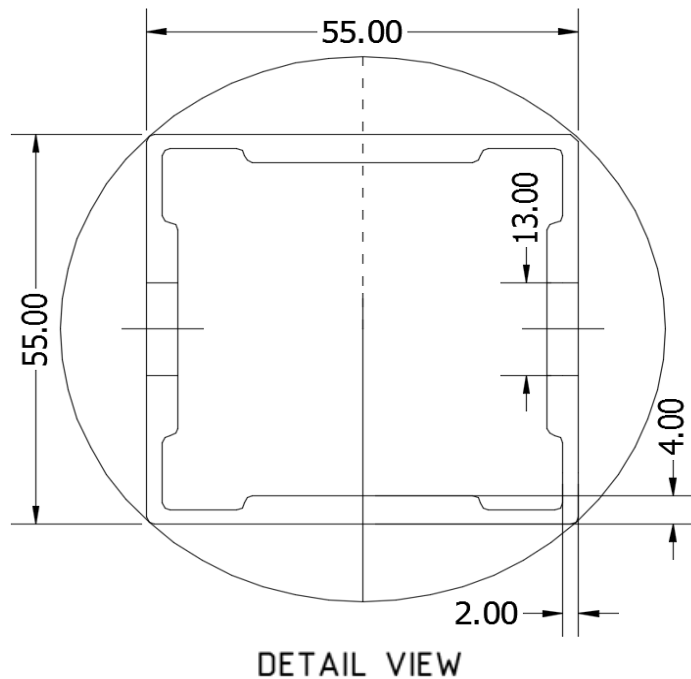
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.100 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.810 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>92%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

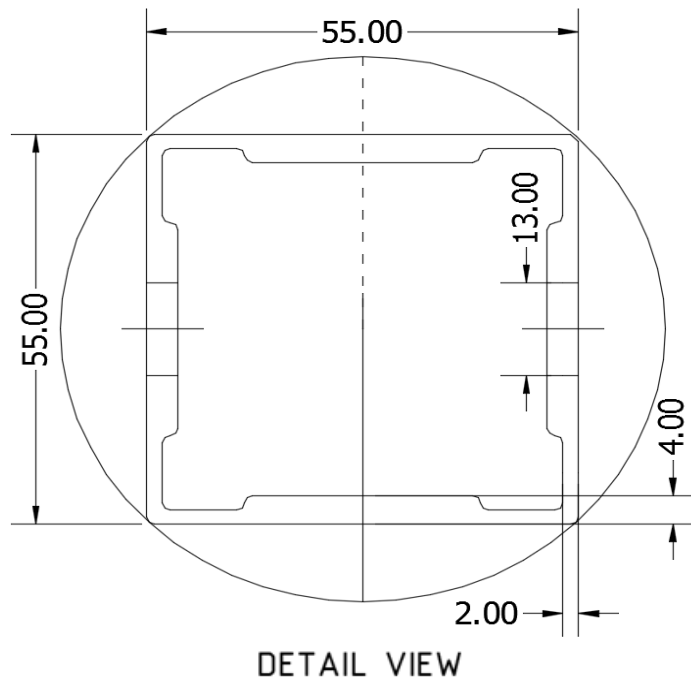
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.666 k-ft
$P_n$ =	0.163 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>48%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.189 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>37%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.135 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<u>37%</u>



## 5. FOUNDATION DESIGN CALCULATIONS

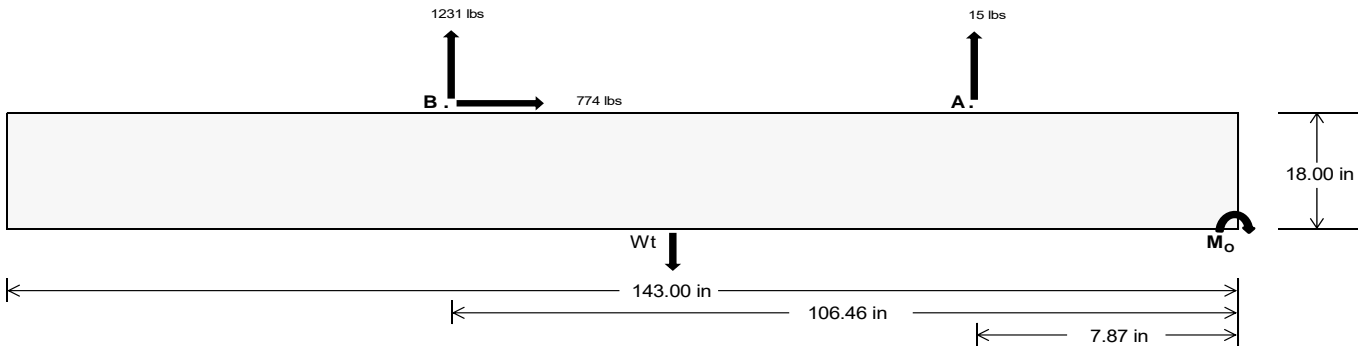
### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =		<u>87.71</u>	<u>5358.80</u> k
Compressive Load =		<u>3663.99</u>	<u>4535.75</u> k
Lateral Load =		<u>435.65</u>	<u>3354.99</u> k
Moment (Weak Axis) =		<u>0.87</u>	<u>0.34</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 145128.6$  in-lbs  
Resisting Force Required = 2029.77 lbs  
S.F. = 1.67  
Weight Required = 3382.95 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Sliding

Force = 773.69 lbs  
Friction = 0.4  
Weight Required = 1934.22 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 773.69 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1398 lbs	1398 lbs	1398 lbs	1398 lbs	1133 lbs	1133 lbs	1133 lbs	1133 lbs	1752 lbs	1752 lbs	1752 lbs	1752 lbs	-29 lbs	-29 lbs	-29 lbs	-29 lbs
$F_B$	1394 lbs	1394 lbs	1394 lbs	1394 lbs	1842 lbs	1842 lbs	1842 lbs	1842 lbs	2283 lbs	2283 lbs	2283 lbs	2283 lbs	-2463 lbs	-2463 lbs	-2463 lbs	-2463 lbs
$F_V$	207 lbs	207 lbs	207 lbs	207 lbs	1418 lbs	1418 lbs	1418 lbs	1418 lbs	1199 lbs	1199 lbs	1199 lbs	1199 lbs	-1547 lbs	-1547 lbs	-1547 lbs	-1547 lbs
$P_{total}$	10352 lbs	10568 lbs	10784 lbs	11000 lbs	10535 lbs	10751 lbs	10967 lbs	11183 lbs	11595 lbs	11811 lbs	12027 lbs	12243 lbs	2044 lbs	2173 lbs	2303 lbs	2433 lbs
$M$	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	2767 lbs-ft	2767 lbs-ft	2767 lbs-ft	2767 lbs-ft	4439 lbs-ft	4439 lbs-ft	4439 lbs-ft	4439 lbs-ft	4698 lbs-ft	4698 lbs-ft	4698 lbs-ft	4698 lbs-ft
$e$	0.35 ft	0.35 ft	0.34 ft	0.33 ft	0.26 ft	0.26 ft	0.25 ft	0.25 ft	0.38 ft	0.38 ft	0.37 ft	0.36 ft	2.30 ft	2.16 ft	2.04 ft	1.93 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	244.8 psf	244.0 psf	243.3 psf	242.6 psf	263.0 psf	261.8 psf	260.6 psf	259.4 psf	269.3 psf	267.8 psf	266.5 psf	265.2 psf	0.0 psf	0.0 psf	0.0 psf	1.8 psf
$f_{max}$	350.9 psf	347.2 psf	343.7 psf	340.4 psf	343.2 psf	339.7 psf	336.4 psf	333.3 psf	397.9 psf	392.9 psf	388.1 psf	383.7 psf	127.6 psf	127.2 psf	127.1 psf	127.1 psf

Maximum Bearing Pressure = 398 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

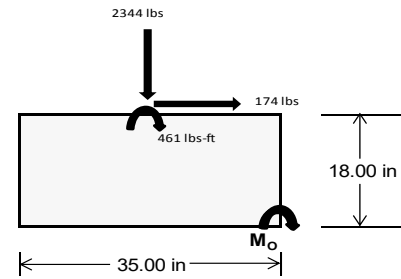
### Overturning Check

$M_o = 2695.9 \text{ ft-lbs}$   
 Resisting Force Required = 1848.62 lbs  
 S.F. = 1.67  
 Weight Required = 3081.03 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	350 lbs	720 lbs	226 lbs	899 lbs	2344 lbs	803 lbs	145 lbs	211 lbs	23 lbs
$F_v$	244 lbs	238 lbs	251 lbs	177 lbs	174 lbs	198 lbs	246 lbs	239 lbs	248 lbs
$P_{total}$	9708 lbs	10079 lbs	9585 lbs	9808 lbs	11253 lbs	9712 lbs	2882 lbs	2947 lbs	2760 lbs
$M$	969 lbs-ft	953 lbs-ft	990 lbs-ft	718 lbs-ft	722 lbs-ft	784 lbs-ft	969 lbs-ft	950 lbs-ft	977 lbs-ft
$e$	0.10 ft	0.09 ft	0.10 ft	0.07 ft	0.06 ft	0.08 ft	0.34 ft	0.32 ft	0.35 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	222.0 psf	233.6 psf	217.2 psf	239.7 psf	281.0 psf	233.1 psf	25.5 psf	28.6 psf	21.6 psf
$f_{max}$	336.7 psf	346.4 psf	334.4 psf	324.7 psf	366.5 psf	325.8 psf	140.3 psf	141.0 psf	137.2 psf



Maximum Bearing Pressure = 366 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 26in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.540 k
Allowable Uplift =	1.214 k
Utilization =	<u>45%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.914 k
Allowable Uplift =	4.357 k
Utilization =	<u>44%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.818 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

#### Rear Strut

Maximum Axial Load =	3.599 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>48%</u>

#### Diagonal Strut

Maximum Axial Load =	2.275 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>31%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	60.93 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	$0.020h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.219 in
	<u><math>0.996 \leq 1.219</math>, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 120 \text{ in}$$

$$J = 0.432$$

$$331.976$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 120$$

$$J = 0.432$$

$$211.117$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.866	-46.866	0	0
2	M14	Y	-46.866	-46.866	0	0
3	M15	Y	-46.866	-46.866	0	0
4	M16	Y	-46.866	-46.866	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-71.679	-71.679	0	0
2	M14	y	-71.679	-71.679	0	0
3	M15	y	-115.31	-115.31	0	0
4	M16	y	-115.31	-115.31	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	162.058	162.058	0	0
2	M14	y	124.66	124.66	0	0
3	M15	y	68.563	68.563	0	0
4	M16	y	68.563	68.563	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 72 Cell 2V 30° 110mph 30psf 10ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	108.144	1	710.123	1	-10.135	12	.004	14	.403	1	1.473	1
20		min	6.376	12	-978.659	3	-231.466	1	-.014	2	.015	12	-1.955	3
21	11	max	108.144	1	585.044	1	-7.815	12	.014	2	.174	1	.753	1
22		min	6.376	12	-804.62	3	-182.243	1	0	3	.005	12	-.964	3
23	12	max	108.144	1	459.965	1	-5.496	12	.014	2	.08	4	.173	1
24		min	6.376	12	-630.582	3	-133.019	1	0	3	-.004	3	-.167	3
25	13	max	108.144	1	334.886	1	-3.177	12	.014	2	.036	5	.437	3
26		min	6.376	12	-456.543	3	-83.796	1	0	3	-.122	1	-.269	1
27	14	max	108.144	1	209.807	1	-.857	12	.014	2	-.002	15	.848	3
28		min	6.376	12	-282.505	3	-41.785	4	0	3	-.188	1	-.572	1
29	15	max	108.144	1	84.727	1	14.651	1	.014	2	-.009	12	1.065	3
30		min	-2.122	5	-108.466	3	-29.662	5	0	3	-.199	1	-.735	1
31	16	max	108.144	1	65.572	3	63.875	1	.014	2	-.006	12	1.089	3
32		min	-15.233	5	-40.352	1	-26.132	5	0	3	-.155	1	-.76	1
33	17	max	108.144	1	239.611	3	113.098	1	.014	2	0	12	.919	3
34		min	-28.345	5	-165.431	1	-22.602	5	0	3	-.111	4	-.646	1
35	18	max	108.144	1	413.649	3	162.321	1	.014	2	.096	1	.556	3
36		min	-41.457	5	-290.51	1	-19.072	5	0	3	-.119	5	-.392	1
37	19	max	108.144	1	587.688	3	211.545	1	.014	2	.304	1	0	1
38		min	-54.568	5	-415.589	1	-15.542	5	0	3	-.138	5	0	3
39	M14	1	max	62.465	4	452.254	1	-11.091	12	.01	.354	1	0	4
40		min	3.214	12	-467.655	3	-219.112	1	-.012	1	.02	12	0	3
41	2	max	58.625	1	327.175	1	-8.772	12	.01	3	.234	4	.446	3
42		min	3.214	12	-335.17	3	-169.889	1	-.012	1	.009	12	-.433	1
43	3	max	58.625	1	202.095	1	-6.453	12	.01	3	.133	5	.745	3
44		min	3.214	12	-202.685	3	-120.666	1	-.012	1	-.023	1	-.727	1
45	4	max	58.625	1	77.016	1	-4.133	12	.01	3	.072	5	.896	3
46		min	3.214	12	-70.2	3	-71.442	1	-.012	1	-.13	1	-.882	1
47	5	max	58.625	1	62.286	3	-1.814	12	.01	3	.015	5	.901	3
48		min	-3.498	5	-48.063	1	-55.532	4	-.012	1	-.182	1	-.898	1
49	6	max	58.625	1	194.771	3	27.005	1	.01	3	-.009	12	.758	3
50		min	-16.61	5	-173.142	1	-45.619	5	-.012	1	-.18	1	-.775	1
51	7	max	58.625	1	327.256	3	76.228	1	.01	3	-.007	12	.468	3
52		min	-29.721	5	-298.221	1	-42.089	5	-.012	1	-.122	1	-.513	1
53	8	max	58.625	1	459.741	3	125.452	1	.01	3	0	10	.031	3
54		min	-42.833	5	-423.3	1	-38.558	5	-.012	1	-.137	4	-.122	2
55	9	max	58.625	1	592.226	3	174.675	1	.01	3	.157	1	.427	1
56		min	-55.945	5	-548.379	1	-35.028	5	-.012	1	-.172	5	-.554	3
57	10	max	90.43	4	673.459	1	-9.783	12	.01	3	.378	1	1.106	1
58		min	3.214	12	-724.711	3	-223.898	1	-.012	1	.014	12	-1.285	3
59	11	max	77.319	4	548.379	1	-7.464	12	.012	1	.234	4	.427	1
60		min	3.214	12	-592.226	3	-174.675	1	-.01	3	.004	12	-.554	3
61	12	max	64.207	4	423.3	1	-5.144	12	.012	1	.13	4	.031	3
62		min	3.214	12	-459.741	3	-125.452	1	-.01	3	-.01	1	-.122	2
63	13	max	58.625	1	298.221	1	-2.825	12	.012	1	.068	5	.468	3
64		min	3.214	12	-327.256	3	-76.228	1	-.01	3	-.122	1	-.513	1
65	14	max	58.625	1	173.142	1	-.506	12	.012	1	.011	5	.758	3
66		min	3.214	12	-194.771	3	-56.692	4	-.01	3	-.18	1	-.775	1
67	15	max	58.625	1	48.063	1	22.219	1	.012	1	-.008	12	.901	3
68		min	3.214	12	-62.286	3	-45.895	5	-.01	3	-.182	1	-.898	1
69	16	max	58.625	1	70.2	3	71.442	1	.012	1	-.005	12	.896	3
70		min	-2.043	5	-77.016	1	-42.365	5	-.01	3	-.13	1	-.882	1
71	17	max	58.625	1	202.685	3	120.666	1	.012	1	.002	3	.745	3
72		min	-15.155	5	-202.095	1	-38.834	5	-.01	3	-.144	4	-.727	1
73	18	max	58.625	1	335.17	3	169.889	1	.012	1	.138	1	.446	3
74		min	-28.266	5	-327.175	1	-35.304	5	-.01	3	-.177	5	-.433	1
75	19	max	58.625	1	467.655	3	219.112	1	.012	1	.354	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-41.378	5	-452.254	1	-31.774	5	-.01	3	-.214	5	0	3
77	M15	1	max	101.605	5	582.839	2	-11.023	12	.013	2	.424	4	0	2
78			min	-62.9	1	-257.645	3	-219.036	1	-.008	3	.02	12	0	3
79		2	max	88.493	5	419.066	2	-8.704	12	.013	2	.289	4	.247	3
80			min	-62.9	1	-187.489	3	-169.812	1	-.008	3	.009	12	-.557	2
81		3	max	75.382	5	255.292	2	-6.384	12	.013	2	.172	5	.417	3
82			min	-62.9	1	-117.334	3	-120.589	1	-.008	3	-.024	1	-.931	2
83		4	max	62.27	5	91.519	2	-4.065	12	.013	2	.096	5	.508	3
84			min	-62.9	1	-47.179	3	-84.265	4	-.008	3	-.13	1	-1.124	2
85		5	max	49.158	5	22.976	3	-1.746	12	.013	2	.024	5	.521	3
86			min	-62.9	1	-72.254	2	-69.35	4	-.008	3	-.182	1	-1.135	2
87		6	max	36.047	5	93.131	3	27.082	1	.013	2	-.009	12	.457	3
88			min	-62.9	1	-236.027	2	-59.374	5	-.008	3	-.18	1	-.963	2
89		7	max	22.935	5	163.287	3	76.305	1	.013	2	-.007	12	.315	3
90			min	-62.9	1	-399.8	2	-55.843	5	-.008	3	-.138	4	-.61	2
91		8	max	9.823	5	233.442	3	125.528	1	.013	2	0	10	.094	3
92			min	-62.9	1	-563.574	2	-52.313	5	-.008	3	-.174	4	-.089	1
93		9	max	-2.071	15	303.597	3	174.752	1	.013	2	.157	1	.642	2
94			min	-62.9	1	-727.347	2	-48.783	5	-.008	3	-.224	5	-.204	3
95		10	max	-3.773	12	891.12	2	-9.851	12	.013	2	.423	4	1.541	2
96			min	-62.9	1	-373.752	3	-223.975	1	-.008	3	.014	12	-.581	3
97		11	max	-3.773	12	727.347	2	-7.532	12	.008	3	.287	4	.642	2
98			min	-62.9	1	-303.597	3	-174.752	1	-.013	2	.005	12	-.204	3
99		12	max	-3.773	12	563.574	2	-5.213	12	.008	3	.167	4	.094	3
100			min	-62.9	1	-233.442	3	-125.528	1	-.013	2	-.01	1	-.089	1
101		13	max	-3.773	12	399.8	2	-2.893	12	.008	3	.09	5	.315	3
102			min	-62.9	1	-163.287	3	-85.481	4	-.013	2	-.122	1	-.61	2
103		14	max	-3.773	12	236.027	2	-.574	12	.008	3	.018	5	.457	3
104			min	-62.9	1	-93.131	3	-70.566	4	-.013	2	-.18	1	-.963	2
105		15	max	-3.773	12	72.254	2	22.142	1	.008	3	-.008	12	.521	3
106			min	-74.692	4	-22.976	3	-59.656	5	-.013	2	-.182	1	-1.135	2
107		16	max	-3.773	12	47.179	3	71.365	1	.008	3	-.005	12	.508	3
108			min	-87.804	4	-91.519	2	-56.126	5	-.013	2	-.147	4	-1.124	2
109		17	max	-3.773	12	117.334	3	120.589	1	.008	3	.001	3	.417	3
110			min	-100.915	4	-255.292	2	-52.596	5	-.013	2	-.184	4	-.931	2
111		18	max	-3.773	12	187.489	3	169.812	1	.008	3	.138	1	.247	3
112			min	-114.027	4	-419.066	2	-49.066	5	-.013	2	-.232	5	-.557	2
113		19	max	-3.773	12	257.645	3	219.036	1	.008	3	.354	1	0	2
114			min	-127.139	4	-582.839	2	-45.536	5	-.013	2	-.284	5	0	5
115	M16	1	max	96.069	5	547.361	2	-10.539	12	.011	1	.317	4	0	2
116			min	-121.703	1	-231.126	3	-212.027	1	-.011	3	.017	12	0	3
117		2	max	82.957	5	383.588	2	-8.22	12	.011	1	.206	4	.218	3
118			min	-121.703	1	-160.971	3	-162.804	1	-.011	3	.007	12	-.517	2
119		3	max	69.846	5	219.815	2	-5.9	12	.011	1	.122	5	.358	3
120			min	-121.703	1	-90.816	3	-113.58	1	-.011	3	-.055	1	-.852	2
121		4	max	56.734	5	56.042	2	-3.581	12	.011	1	.068	5	.42	3
122			min	-121.703	1	-20.661	3	-64.357	1	-.011	3	-.154	1	-1.006	2
123		5	max	43.622	5	49.494	3	-1.261	12	.011	1	.018	5	.404	3
124			min	-121.703	1	-107.732	2	-47.686	4	-.011	3	-.198	1	-.977	2
125		6	max	30.511	5	119.649	3	34.09	1	.011	1	-.009	12	.31	3
126			min	-121.703	1	-271.505	2	-39.726	5	-.011	3	-.188	1	-.766	2
127		7	max	17.399	5	189.805	3	83.314	1	.011	1	-.007	12	.138	3
128			min	-121.703	1	-435.278	2	-36.196	5	-.011	3	-.122	1	-.374	2
129		8	max	4.287	5	259.96	3	132.537	1	.011	1	0	10	.201	2
130			min	-121.703	1	-599.051	2	-32.665	5	-.011	3	-.112	4	-.112	3
131		9	max	-5.809	15	330.115	3	181.76	1	.011	1	.172	1	.958	2
132			min	-121.703	1	-762.824	2	-29.135	5	-.011	3	-.143	5	-.44	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-6.821	12	926.597	2	-10.335	12	.011	1	.402	1	1.896	2
134			min	-121.703	1	-400.27	3	-230.984	1	-.011	3	.016	12	-.846	3
135		11	max	-5.497	15	762.824	2	-8.016	12	.011	3	.211	4	.958	2
136			min	-121.703	1	-330.115	3	-181.76	1	-.011	1	.006	12	-.44	3
137		12	max	-6.821	12	599.051	2	-5.697	12	.011	3	.11	4	.201	2
138			min	-121.703	1	-259.96	3	-132.537	1	-.011	1	-.003	3	-.112	3
139		13	max	-6.821	12	435.278	2	-3.377	12	.011	3	.053	5	.138	3
140			min	-121.703	1	-189.805	3	-83.314	1	-.011	1	-.122	1	-.374	2
141		14	max	-6.821	12	271.505	2	-1.058	12	.011	3	.002	5	.31	3
142			min	-121.703	1	-119.649	3	-53.224	4	-.011	1	-.188	1	-.766	2
143		15	max	-6.821	12	107.732	2	15.133	1	.011	3	-.009	12	.404	3
144			min	-121.703	1	-49.494	3	-41.052	5	-.011	1	-.198	1	-.977	2
145		16	max	-6.821	12	20.661	3	64.357	1	.011	3	-.006	12	.42	3
146			min	-121.703	1	-56.042	2	-37.522	5	-.011	1	-.154	1	-1.006	2
147		17	max	-6.821	12	90.816	3	113.58	1	.011	3	-.001	12	.358	3
148			min	-121.703	1	-219.815	2	-33.992	5	-.011	1	-.144	4	-.852	2
149		18	max	-6.821	12	160.971	3	162.804	1	.011	3	.098	1	.218	3
150			min	-129.875	4	-383.588	2	-30.462	5	-.011	1	-.165	5	-.517	2
151		19	max	-6.821	12	231.126	3	212.027	1	.011	3	.307	1	0	2
152			min	-142.987	4	-547.361	2	-26.931	5	-.011	1	-.197	5	0	5
153	M2	1	max	987.235	1	2.056	4	.605	1	0	12	0	3	0	1
154			min	-1112.805	3	.498	15	-37.161	4	0	4	0	1	0	1
155		2	max	987.764	1	1.985	4	.605	1	0	12	0	1	0	15
156			min	-1112.408	3	.481	15	-37.623	4	0	4	-.013	4	0	4
157		3	max	988.294	1	1.914	4	.605	1	0	12	0	1	0	15
158			min	-1112.011	3	.464	15	-38.084	4	0	4	-.027	4	-.001	4
159		4	max	988.823	1	1.843	4	.605	1	0	12	0	1	0	15
160			min	-1111.614	3	.448	15	-38.545	4	0	4	-.041	4	-.002	4
161		5	max	989.352	1	1.772	4	.605	1	0	12	0	1	0	15
162			min	-1111.217	3	.431	15	-39.006	4	0	4	-.055	4	-.003	4
163		6	max	989.881	1	1.701	4	.605	1	0	12	.001	1	0	15
164			min	-1110.82	3	.414	15	-39.467	4	0	4	-.069	4	-.003	4
165		7	max	990.411	1	1.63	4	.605	1	0	12	.001	1	0	15
166			min	-1110.423	3	.398	15	-39.929	4	0	4	-.083	4	-.004	4
167		8	max	990.94	1	1.559	4	.605	1	0	12	.002	1	-.001	15
168			min	-1110.026	3	.381	15	-40.39	4	0	4	-.097	4	-.005	4
169		9	max	991.469	1	1.488	4	.605	1	0	12	.002	1	-.001	15
170			min	-1109.629	3	.364	15	-40.851	4	0	4	-.112	4	-.005	4
171		10	max	991.999	1	1.417	4	.605	1	0	12	.002	1	-.001	15
172			min	-1109.232	3	.348	15	-41.312	4	0	4	-.127	4	-.006	4
173		11	max	992.528	1	1.345	4	.605	1	0	12	.002	1	-.001	15
174			min	-1108.835	3	.331	15	-41.774	4	0	4	-.142	4	-.006	4
175		12	max	993.057	1	1.274	4	.605	1	0	12	.002	1	-.002	15
176			min	-1108.438	3	.314	15	-42.235	4	0	4	-.157	4	-.007	4
177		13	max	993.586	1	1.203	4	.605	1	0	12	.003	1	-.002	15
178			min	-1108.041	3	.297	15	-42.696	4	0	4	-.172	4	-.007	4
179		14	max	994.116	1	1.132	4	.605	1	0	12	.003	1	-.002	15
180			min	-1107.644	3	.281	15	-43.157	4	0	4	-.187	4	-.007	4
181		15	max	994.645	1	1.061	4	.605	1	0	12	.003	1	-.002	15
182			min	-1107.248	3	.262	12	-43.618	4	0	4	-.203	4	-.008	4
183		16	max	995.174	1	.99	4	.605	1	0	12	.003	1	-.002	15
184			min	-1106.851	3	.235	12	-44.08	4	0	4	-.219	4	-.008	4
185		17	max	995.704	1	.919	4	.605	1	0	12	.003	1	-.002	15
186			min	-1106.454	3	.207	12	-44.541	4	0	4	-.234	4	-.009	4
187		18	max	996.233	1	.848	4	.605	1	0	12	.004	1	-.002	15
188			min	-1106.057	3	.179	12	-45.002	4	0	4	-.251	4	-.009	4
189		19	max	996.762	1	.777	4	.605	1	0	12	.004	1	-.002	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1105.66	3	.152	12	-45.463	4	0	4	-.267	4	-.009	4
191	M3	1	max	576.657	2	8.9	4	1.931	4	0	12	0	1	.009	4
192			min	-734.806	3	2.103	15	.026	12	0	4	-.029	4	.002	15
193		2	max	576.487	2	8.031	4	2.536	4	0	12	0	1	.005	4
194			min	-734.933	3	1.899	15	.026	12	0	4	-.028	4	.001	12
195		3	max	576.316	2	7.162	4	3.141	4	0	12	0	1	.002	2
196			min	-735.061	3	1.695	15	.026	12	0	4	-.026	4	0	3
197		4	max	576.146	2	6.293	4	3.746	4	0	12	.001	1	0	15
198			min	-735.189	3	1.49	15	.026	12	0	4	-.025	4	-.002	3
199		5	max	575.976	2	5.424	4	4.351	4	0	12	.001	1	0	15
200			min	-735.317	3	1.286	15	.026	12	0	4	-.023	4	-.004	6
201		6	max	575.805	2	4.555	4	4.956	4	0	12	.002	1	-.002	15
202			min	-735.444	3	1.082	15	.026	12	0	4	-.021	5	-.007	6
203		7	max	575.635	2	3.686	4	5.561	4	0	12	.002	1	-.002	15
204			min	-735.572	3	.878	15	.026	12	0	4	-.018	5	-.009	6
205		8	max	575.465	2	2.818	4	6.166	4	0	12	.002	1	-.002	15
206			min	-735.7	3	.673	15	.026	12	0	4	-.016	5	-.01	6
207		9	max	575.294	2	1.949	4	6.771	4	0	12	.002	1	-.003	15
208			min	-735.828	3	.469	15	.026	12	0	4	-.013	5	-.011	6
209		10	max	575.124	2	1.08	4	7.377	4	0	12	.002	1	-.003	15
210			min	-735.955	3	.265	15	.026	12	0	4	-.009	5	-.012	6
211		11	max	574.954	2	.271	2	7.982	4	0	12	.003	1	-.003	15
212			min	-736.083	3	-.067	3	.026	12	0	4	-.006	5	-.012	6
213		12	max	574.783	2	-.144	15	8.587	4	0	12	.003	1	-.003	15
214			min	-736.211	3	-.659	6	.026	12	0	4	-.002	5	-.012	6
215		13	max	574.613	2	-.348	15	9.192	4	0	12	.003	1	-.003	15
216			min	-736.339	3	-1.528	6	.026	12	0	4	0	12	-.012	6
217		14	max	574.443	2	-.552	15	9.797	4	0	12	.007	4	-.003	15
218			min	-736.467	3	-2.397	6	.026	12	0	4	0	12	-.011	6
219		15	max	574.272	2	-.756	15	10.402	4	0	12	.012	4	-.002	15
220			min	-736.594	3	-3.266	6	.026	12	0	4	0	12	-.009	6
221		16	max	574.102	2	-.961	15	11.007	4	0	12	.017	4	-.002	15
222			min	-736.722	3	-4.135	6	.026	12	0	4	0	12	-.008	6
223		17	max	573.932	2	-1.165	15	11.612	4	0	12	.022	4	-.001	15
224			min	-736.85	3	-5.003	6	.026	12	0	4	0	12	-.006	6
225		18	max	573.761	2	-1.369	15	12.217	4	0	12	.028	4	0	15
226			min	-736.978	3	-5.872	6	.026	12	0	4	0	12	-.003	6
227		19	max	573.591	2	-1.573	15	12.822	4	0	12	.034	4	0	1
228			min	-737.105	3	-6.741	6	.026	12	0	4	0	12	0	1
229	M4	1	max	1105.103	1	0	1	-1.068	12	0	1	.027	4	0	1
230			min	-57.435	5	0	1	-334.056	4	0	1	0	12	0	1
231		2	max	1105.273	1	0	1	-1.068	12	0	1	.001	1	0	1
232			min	-57.356	5	0	1	-334.204	4	0	1	-.011	4	0	1
233		3	max	1105.444	1	0	1	-1.068	12	0	1	0	12	0	1
234			min	-57.276	5	0	1	-334.352	4	0	1	-.05	4	0	1
235		4	max	1105.614	1	0	1	-1.068	12	0	1	0	12	0	1
236			min	-57.197	5	0	1	-334.499	4	0	1	-.088	4	0	1
237		5	max	1105.784	1	0	1	-1.068	12	0	1	0	12	0	1
238			min	-57.117	5	0	1	-334.647	4	0	1	-.126	4	0	1
239		6	max	1105.955	1	0	1	-1.068	12	0	1	0	12	0	1
240			min	-57.038	5	0	1	-334.795	4	0	1	-.165	4	0	1
241		7	max	1106.125	1	0	1	-1.068	12	0	1	0	12	0	1
242			min	-56.958	5	0	1	-334.942	4	0	1	-.203	4	0	1
243		8	max	1106.295	1	0	1	-1.068	12	0	1	0	12	0	1
244			min	-56.879	5	0	1	-335.09	4	0	1	-.242	4	0	1
245		9	max	1106.466	1	0	1	-1.068	12	0	1	0	12	0	1
246			min	-56.799	5	0	1	-335.238	4	0	1	-.28	4	0	1





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1106.636	1	0	1	-1.068	12	0	1	0	12	0	1
248		min	-56.72	5	0	1	-335.385	4	0	1	-.319	4	0	1
249	11	max	1106.806	1	0	1	-1.068	12	0	1	-.001	12	0	1
250		min	-56.64	5	0	1	-335.533	4	0	1	-.357	4	0	1
251	12	max	1106.977	1	0	1	-1.068	12	0	1	-.001	12	0	1
252		min	-56.561	5	0	1	-335.68	4	0	1	-.396	4	0	1
253	13	max	1107.147	1	0	1	-1.068	12	0	1	-.001	12	0	1
254		min	-56.481	5	0	1	-335.828	4	0	1	-.434	4	0	1
255	14	max	1107.317	1	0	1	-1.068	12	0	1	-.001	12	0	1
256		min	-56.402	5	0	1	-335.976	4	0	1	-.473	4	0	1
257	15	max	1107.488	1	0	1	-1.068	12	0	1	-.002	12	0	1
258		min	-56.322	5	0	1	-336.123	4	0	1	-.511	4	0	1
259	16	max	1107.658	1	0	1	-1.068	12	0	1	-.002	12	0	1
260		min	-56.243	5	0	1	-336.271	4	0	1	-.55	4	0	1
261	17	max	1107.828	1	0	1	-1.068	12	0	1	-.002	12	0	1
262		min	-56.163	5	0	1	-336.419	4	0	1	-.589	4	0	1
263	18	max	1107.999	1	0	1	-1.068	12	0	1	-.002	12	0	1
264		min	-56.084	5	0	1	-336.566	4	0	1	-.627	4	0	1
265	19	max	1108.169	1	0	1	-1.068	12	0	1	-.002	12	0	1
266		min	-56.004	5	0	1	-336.714	4	0	1	-.666	4	0	1
267	M6	1	max	3125.705	1	2.173	2	0	1	0	0	4	0	1
268		min	-3599.169	3	.352	12	-37.604	4	0	4	0	1	0	1
269	2	max	3126.235	1	2.118	2	0	1	0	1	0	1	0	12
270		min	-3598.772	3	.325	12	-38.065	4	0	4	-.014	4	0	2
271	3	max	3126.764	1	2.062	2	0	1	0	1	0	1	0	12
272		min	-3598.375	3	.297	12	-38.526	4	0	4	-.027	4	-.002	2
273	4	max	3127.293	1	2.007	2	0	1	0	1	0	1	0	12
274		min	-3597.978	3	.269	12	-38.987	4	0	4	-.041	4	-.002	2
275	5	max	3127.822	1	1.951	2	0	1	0	1	0	1	0	12
276		min	-3597.581	3	.242	12	-39.449	4	0	4	-.055	4	-.003	2
277	6	max	3128.352	1	1.896	2	0	1	0	1	0	1	0	12
278		min	-3597.184	3	.214	12	-39.91	4	0	4	-.07	4	-.004	2
279	7	max	3128.881	1	1.841	2	0	1	0	1	0	1	0	12
280		min	-3596.787	3	.186	12	-40.371	4	0	4	-.084	4	-.004	2
281	8	max	3129.41	1	1.785	2	0	1	0	1	0	1	0	12
282		min	-3596.39	3	.159	12	-40.832	4	0	4	-.098	4	-.005	2
283	9	max	3129.94	1	1.73	2	0	1	0	1	0	1	0	12
284		min	-3595.993	3	.123	3	-41.293	4	0	4	-.113	4	-.006	2
285	10	max	3130.469	1	1.675	2	0	1	0	1	0	1	0	12
286		min	-3595.596	3	.081	3	-41.755	4	0	4	-.128	4	-.006	2
287	11	max	3130.998	1	1.619	2	0	1	0	1	0	1	0	12
288		min	-3595.199	3	.04	3	-42.216	4	0	4	-.143	4	-.007	2
289	12	max	3131.527	1	1.564	2	0	1	0	1	0	1	0	12
290		min	-3594.802	3	-.002	3	-42.677	4	0	4	-.158	4	-.007	2
291	13	max	3132.057	1	1.509	2	0	1	0	1	0	1	0	12
292		min	-3594.405	3	-.043	3	-43.138	4	0	4	-.174	4	-.008	2
293	14	max	3132.586	1	1.453	2	0	1	0	1	0	1	0	12
294		min	-3594.008	3	-.085	3	-43.6	4	0	4	-.189	4	-.008	2
295	15	max	3133.115	1	1.398	2	0	1	0	1	0	1	0	12
296		min	-3593.611	3	-.126	3	-44.061	4	0	4	-.205	4	-.009	2
297	16	max	3133.645	1	1.343	2	0	1	0	1	0	1	0	3
298		min	-3593.214	3	-.168	3	-44.522	4	0	4	-.221	4	-.009	2
299	17	max	3134.174	1	1.287	2	0	1	0	1	0	1	0	3
300		min	-3592.817	3	-.209	3	-44.983	4	0	4	-.237	4	-.01	2
301	18	max	3134.703	1	1.232	2	0	1	0	1	0	1	0	3
302		min	-3592.42	3	-.251	3	-45.444	4	0	4	-.253	4	-.01	2
303	19	max	3135.232	1	1.177	2	0	1	0	1	0	1	0	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-3592.023	3	-.292	3	-45.906	4	0	4	-.27	4	-.011	2
305	M7	1	max	2188.956	2	8.909	6	1.399	4	0	1	0	1	.011	2
306			min	-2272.423	3	2.091	15	0	1	0	4	-.029	4	0	3
307		2	max	2188.786	2	8.04	6	2.004	4	0	1	0	1	.008	2
308			min	-2272.551	3	1.887	15	0	1	0	4	-.028	4	-.002	3
309		3	max	2188.615	2	7.171	6	2.609	4	0	1	0	1	.005	2
310			min	-2272.678	3	1.683	15	0	1	0	4	-.027	4	-.003	3
311		4	max	2188.445	2	6.302	6	3.215	4	0	1	0	1	.002	2
312			min	-2272.806	3	1.479	15	0	1	0	4	-.026	4	-.005	3
313		5	max	2188.275	2	5.433	6	3.82	4	0	1	0	1	0	2
314			min	-2272.934	3	1.274	15	0	1	0	4	-.024	4	-.006	3
315		6	max	2188.104	2	4.564	6	4.425	4	0	1	0	1	-.002	15
316			min	-2273.062	3	1.07	15	0	1	0	4	-.022	4	-.007	3
317		7	max	2187.934	2	3.695	6	5.03	4	0	1	0	1	-.002	15
318			min	-2273.189	3	.866	15	0	1	0	4	-.02	4	-.009	4
319		8	max	2187.764	2	2.826	6	5.635	4	0	1	0	1	-.002	15
320			min	-2273.317	3	.662	15	0	1	0	4	-.017	4	-.01	4
321		9	max	2187.593	2	1.957	6	6.24	4	0	1	0	1	-.003	15
322			min	-2273.445	3	.417	12	0	1	0	4	-.015	4	-.011	4
323		10	max	2187.423	2	1.277	2	6.845	4	0	1	0	1	-.003	15
324			min	-2273.573	3	.062	3	0	1	0	4	-.011	4	-.012	4
325		11	max	2187.253	2	.6	2	7.45	4	0	1	0	1	-.003	15
326			min	-2273.701	3	-.446	3	0	1	0	4	-.008	4	-.012	4
327		12	max	2187.082	2	-.078	2	8.055	4	0	1	0	1	-.003	15
328			min	-2273.828	3	-.954	3	0	1	0	4	-.004	4	-.012	4
329		13	max	2186.912	2	-.359	15	8.66	4	0	1	0	1	-.003	15
330			min	-2273.956	3	-1.518	4	0	1	0	4	0	4	-.012	4
331		14	max	2186.742	2	-.564	15	9.265	4	0	1	.004	4	-.003	15
332			min	-2274.084	3	-2.387	4	0	1	0	4	0	1	-.011	4
333		15	max	2186.571	2	-.768	15	9.87	4	0	1	.008	4	-.002	15
334			min	-2274.212	3	-3.256	4	0	1	0	4	0	1	-.009	4
335		16	max	2186.401	2	-.972	15	10.475	4	0	1	.013	4	-.002	15
336			min	-2274.339	3	-4.125	4	0	1	0	4	0	1	-.008	4
337		17	max	2186.231	2	-1.176	15	11.08	4	0	1	.018	4	-.001	15
338			min	-2274.467	3	-4.994	4	0	1	0	4	0	1	-.006	4
339		18	max	2186.06	2	-1.381	15	11.686	4	0	1	.023	4	0	15
340			min	-2274.595	3	-5.863	4	0	1	0	4	0	1	-.003	4
341		19	max	2185.89	2	-1.585	15	12.291	4	0	1	.029	4	0	1
342			min	-2274.723	3	-6.732	4	0	1	0	4	0	1	0	1
343	M8	1	max	2815.391	1	0	1	0	1	0	1	.023	4	0	1
344			min	-69.768	3	0	1	-317.409	4	0	1	0	1	0	1
345		2	max	2815.561	1	0	1	0	1	0	1	0	1	0	1
346			min	-69.64	3	0	1	-317.556	4	0	1	-.013	4	0	1
347		3	max	2815.732	1	0	1	0	1	0	1	0	1	0	1
348			min	-69.512	3	0	1	-317.704	4	0	1	-.05	4	0	1
349		4	max	2815.902	1	0	1	0	1	0	1	0	1	0	1
350			min	-69.385	3	0	1	-317.852	4	0	1	-.086	4	0	1
351		5	max	2816.072	1	0	1	0	1	0	1	0	1	0	1
352			min	-69.257	3	0	1	-317.999	4	0	1	-.123	4	0	1
353		6	max	2816.243	1	0	1	0	1	0	1	0	1	0	1
354			min	-69.129	3	0	1	-318.147	4	0	1	-.159	4	0	1
355		7	max	2816.413	1	0	1	0	1	0	1	0	1	0	1
356			min	-69.001	3	0	1	-318.295	4	0	1	-.196	4	0	1
357		8	max	2816.583	1	0	1	0	1	0	1	0	1	0	1
358			min	-68.874	3	0	1	-318.442	4	0	1	-.232	4	0	1
359		9	max	2816.754	1	0	1	0	1	0	1	0	1	0	1
360			min	-68.746	3	0	1	-318.59	4	0	1	-.269	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2816.924	1	0	1	0	1	0	1	0	1	0	1
362			min	-68.618	3	0	1	-318.738	4	0	1	-.305	4	0	1
363		11	max	2817.094	1	0	1	0	1	0	1	0	1	0	1
364			min	-68.49	3	0	1	-318.885	4	0	1	-.342	4	0	1
365		12	max	2817.265	1	0	1	0	1	0	1	0	1	0	1
366			min	-68.363	3	0	1	-319.033	4	0	1	-.379	4	0	1
367		13	max	2817.435	1	0	1	0	1	0	1	0	1	0	1
368			min	-68.235	3	0	1	-319.18	4	0	1	-.415	4	0	1
369		14	max	2817.605	1	0	1	0	1	0	1	0	1	0	1
370			min	-68.107	3	0	1	-319.328	4	0	1	-.452	4	0	1
371		15	max	2817.776	1	0	1	0	1	0	1	0	1	0	1
372			min	-67.979	3	0	1	-319.476	4	0	1	-.489	4	0	1
373		16	max	2817.946	1	0	1	0	1	0	1	0	1	0	1
374			min	-67.851	3	0	1	-319.623	4	0	1	-.525	4	0	1
375		17	max	2818.117	1	0	1	0	1	0	1	0	1	0	1
376			min	-67.724	3	0	1	-319.771	4	0	1	-.562	4	0	1
377		18	max	2818.287	1	0	1	0	1	0	1	0	1	0	1
378			min	-67.596	3	0	1	-319.919	4	0	1	-.599	4	0	1
379		19	max	2818.457	1	0	1	0	1	0	1	0	1	0	1
380			min	-67.468	3	0	1	-320.066	4	0	1	-.635	4	0	1
381	M10	1	max	987.235	1	1.99	6	-.034	12	0	1	0	4	0	1
382			min	-1112.805	3	.453	15	-37.53	4	0	5	0	3	0	1
383		2	max	987.764	1	1.919	6	-.034	12	0	1	0	10	0	15
384			min	-1112.408	3	.437	15	-37.992	4	0	5	-.014	4	0	6
385		3	max	988.294	1	1.848	6	-.034	12	0	1	0	12	0	15
386			min	-1112.011	3	.42	15	-38.453	4	0	5	-.027	4	-.001	6
387		4	max	988.823	1	1.777	6	-.034	12	0	1	0	12	0	15
388			min	-1111.614	3	.403	15	-38.914	4	0	5	-.041	4	-.002	6
389		5	max	989.352	1	1.706	6	-.034	12	0	1	0	12	0	15
390			min	-1111.217	3	.387	15	-39.375	4	0	5	-.055	4	-.003	6
391		6	max	989.881	1	1.634	6	-.034	12	0	1	0	12	0	15
392			min	-1110.82	3	.37	15	-39.836	4	0	5	-.069	4	-.003	6
393		7	max	990.411	1	1.563	6	-.034	12	0	1	0	12	0	15
394			min	-1110.423	3	.353	15	-40.298	4	0	5	-.084	4	-.004	6
395		8	max	990.94	1	1.492	6	-.034	12	0	1	0	12	0	15
396			min	-1110.026	3	.337	15	-40.759	4	0	5	-.098	4	-.004	6
397		9	max	991.469	1	1.421	6	-.034	12	0	1	0	12	-.001	15
398			min	-1109.629	3	.32	15	-41.22	4	0	5	-.113	4	-.005	6
399		10	max	991.999	1	1.35	6	-.034	12	0	1	0	12	-.001	15
400			min	-1109.232	3	.303	15	-41.681	4	0	5	-.128	4	-.005	6
401		11	max	992.528	1	1.279	6	-.034	12	0	1	0	12	-.001	15
402			min	-1108.835	3	.286	15	-42.143	4	0	5	-.143	4	-.006	6
403		12	max	993.057	1	1.208	6	-.034	12	0	1	0	12	-.001	15
404			min	-1108.438	3	.27	15	-42.604	4	0	5	-.158	4	-.006	6
405		13	max	993.586	1	1.137	6	-.034	12	0	1	0	12	-.002	15
406			min	-1108.041	3	.253	15	-43.065	4	0	5	-.173	4	-.007	6
407		14	max	994.116	1	1.066	6	-.034	12	0	1	0	12	-.002	15
408			min	-1107.644	3	.236	15	-43.526	4	0	5	-.189	4	-.007	6
409		15	max	994.645	1	.995	6	-.034	12	0	1	0	12	-.002	15
410			min	-1107.248	3	.22	15	-43.987	4	0	5	-.205	4	-.007	6
411		16	max	995.174	1	.924	6	-.034	12	0	1	0	12	-.002	15
412			min	-1106.851	3	.203	15	-44.449	4	0	5	-.221	4	-.008	6
413		17	max	995.704	1	.857	2	-.034	12	0	1	0	12	-.002	15
414			min	-1106.454	3	.186	15	-44.91	4	0	5	-.237	4	-.008	6
415		18	max	996.233	1	.801	2	-.034	12	0	1	0	12	-.002	15
416			min	-1106.057	3	.17	15	-45.371	4	0	5	-.253	4	-.008	6
417		19	max	996.762	1	.746	2	-.034	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418	M11	min	-1105.66	3	.152	12	-45.832	4	0	5	-.269	4	-.009	6
419		max	576.657	2	8.849	6	1.645	5	0	1	0	12	.009	6
420		min	-734.806	3	2.069	15	-.472	1	0	4	-.029	4	.002	15
421		2 max	576.487	2	7.98	6	2.251	5	0	1	0	12	.005	2
422		min	-734.933	3	1.865	15	-.472	1	0	4	-.028	4	.001	15
423		3 max	576.316	2	7.112	6	2.856	5	0	1	0	12	.002	2
424		min	-735.061	3	1.661	15	-.472	1	0	4	-.027	4	0	3
425		4 max	576.146	2	6.243	6	3.461	5	0	1	0	12	0	2
426		min	-735.189	3	1.457	15	-.472	1	0	4	-.025	4	-.002	3
427		5 max	575.976	2	5.374	6	4.066	5	0	1	0	12	-.001	15
428		min	-735.317	3	1.252	15	-.472	1	0	4	-.023	4	-.005	4
429		6 max	575.805	2	4.505	6	4.671	5	0	1	0	12	-.002	15
430		min	-735.444	3	1.048	15	-.472	1	0	4	-.021	4	-.007	4
431		7 max	575.635	2	3.636	6	5.276	5	0	1	0	12	-.002	15
432		min	-735.572	3	.844	15	-.472	1	0	4	-.019	4	-.009	4
433		8 max	575.465	2	2.767	6	5.881	5	0	1	0	12	-.003	15
434		min	-735.7	3	.64	15	-.472	1	0	4	-.016	4	-.01	4
435	M12	9 max	575.294	2	1.898	6	6.486	5	0	1	0	12	-.003	15
436		min	-735.828	3	.435	15	-.472	1	0	4	-.014	4	-.012	4
437		10 max	575.124	2	1.029	6	7.091	5	0	1	0	12	-.003	15
438		min	-735.955	3	.231	15	-.472	1	0	4	-.01	4	-.012	4
439		11 max	574.954	2	.271	2	7.696	5	0	1	0	12	-.003	15
440		min	-736.083	3	-.067	3	-.472	1	0	4	-.007	4	-.012	4
441		12 max	574.783	2	-.177	15	8.301	5	0	1	0	12	-.003	15
442		min	-736.211	3	-.709	4	-.472	1	0	4	-.003	4	-.012	4
443		13 max	574.613	2	-.382	15	8.906	5	0	1	.002	5	-.003	15
444		min	-736.339	3	-1.578	4	-.472	1	0	4	-.003	1	-.012	4
445		14 max	574.443	2	-.586	15	9.511	5	0	1	.006	5	-.003	15
446		min	-736.467	3	-2.447	4	-.472	1	0	4	-.003	1	-.011	4
447		15 max	574.272	2	-.79	15	10.116	5	0	1	.011	5	-.002	15
448		min	-736.594	3	-3.316	4	-.472	1	0	4	-.004	1	-.01	4
449		16 max	574.102	2	-.994	15	10.722	5	0	1	.015	5	-.002	15
450		min	-736.722	3	-4.185	4	-.472	1	0	4	-.004	1	-.008	4
451		17 max	573.932	2	-1.199	15	11.327	5	0	1	.021	5	-.001	15
452		min	-736.85	3	-5.054	4	-.472	1	0	4	-.004	1	-.006	4
453	M12	18 max	573.761	2	-1.403	15	11.932	5	0	1	.026	5	0	15
454		min	-736.978	3	-5.923	4	-.472	1	0	4	-.004	1	-.003	4
455		19 max	573.591	2	-1.607	15	12.537	5	0	1	.032	5	0	1
456		min	-737.105	3	-6.792	4	-.472	1	0	4	-.004	1	0	1
457		1 max	1105.103	1	0	1	19.328	1	0	1	.026	5	0	1
458		min	8.967	3	0	1	-322.21	4	0	1	-.004	1	0	1
459		2 max	1105.273	1	0	1	19.328	1	0	1	0	12	0	1
460		min	9.095	3	0	1	-322.357	4	0	1	-.012	4	0	1
461		3 max	1105.444	1	0	1	19.328	1	0	1	0	1	0	1
462		min	9.223	3	0	1	-322.505	4	0	1	-.049	4	0	1
463		4 max	1105.614	1	0	1	19.328	1	0	1	.003	1	0	1
464		min	9.35	3	0	1	-322.653	4	0	1	-.086	4	0	1
465		5 max	1105.784	1	0	1	19.328	1	0	1	.005	1	0	1
466		min	9.478	3	0	1	-322.8	4	0	1	-.123	4	0	1
467		6 max	1105.955	1	0	1	19.328	1	0	1	.008	1	0	1
468		min	9.606	3	0	1	-322.948	4	0	1	-.16	4	0	1
469		7 max	1106.125	1	0	1	19.328	1	0	1	.01	1	0	1
470		min	9.734	3	0	1	-323.096	4	0	1	-.197	4	0	1
471	M12	8 max	1106.295	1	0	1	19.328	1	0	1	.012	1	0	1
472		min	9.861	3	0	1	-323.243	4	0	1	-.234	4	0	1
473		9 max	1106.466	1	0	1	19.328	1	0	1	.014	1	0	1
474		min	9.989	3	0	1	-323.391	4	0	1	-.271	4	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1106.636	1	0	1	19.328	1	0	1	.016	1	0	1
476		min	10.117	3	0	1	-323.539	4	0	1	-.309	4	0	1
477	11	max	1106.806	1	0	1	19.328	1	0	1	.019	1	0	1
478		min	10.245	3	0	1	-323.686	4	0	1	-.346	4	0	1
479	12	max	1106.977	1	0	1	19.328	1	0	1	.021	1	0	1
480		min	10.372	3	0	1	-323.834	4	0	1	-.383	4	0	1
481	13	max	1107.147	1	0	1	19.328	1	0	1	.023	1	0	1
482		min	10.5	3	0	1	-323.981	4	0	1	-.42	4	0	1
483	14	max	1107.317	1	0	1	19.328	1	0	1	.025	1	0	1
484		min	10.628	3	0	1	-324.129	4	0	1	-.457	4	0	1
485	15	max	1107.488	1	0	1	19.328	1	0	1	.028	1	0	1
486		min	10.756	3	0	1	-324.277	4	0	1	-.495	4	0	1
487	16	max	1107.658	1	0	1	19.328	1	0	1	.03	1	0	1
488		min	10.883	3	0	1	-324.424	4	0	1	-.532	4	0	1
489	17	max	1107.828	1	0	1	19.328	1	0	1	.032	1	0	1
490		min	11.011	3	0	1	-324.572	4	0	1	-.569	4	0	1
491	18	max	1107.999	1	0	1	19.328	1	0	1	.034	1	0	1
492		min	11.139	3	0	1	-324.72	4	0	1	-.606	4	0	1
493	19	max	1108.169	1	0	1	19.328	1	0	1	.036	1	0	1
494		min	11.267	3	0	1	-324.867	4	0	1	-.644	4	0	1
495	M1	1	max	211.552	1	587.636	3	54.5	5	0	.304	1	0	3
496		min	-15.542	5	-413.278	1	-107.925	1	0	3	-.138	5	-.014	2
497	2	max	212.395	1	586.542	3	55.961	5	0	1	.237	1	.243	1
498		min	-15.149	5	-414.737	1	-107.925	1	0	3	-.104	5	-.365	3
499	3	max	471.337	3	481.346	1	21.5	5	0	3	.17	1	.491	1
500		min	-289.887	2	-434.008	3	-107.653	1	0	1	-.069	5	-.717	3
501	4	max	471.969	3	479.887	1	22.96	5	0	3	.103	1	.193	1
502		min	-289.044	2	-435.103	3	-107.653	1	0	1	-.055	5	-.447	3
503	5	max	472.601	3	478.428	1	24.42	5	0	3	.036	1	-.005	15
504		min	-288.202	2	-436.197	3	-107.653	1	0	1	-.041	5	-.177	3
505	6	max	473.232	3	476.969	1	25.88	5	0	3	-.002	12	.094	3
506		min	-287.36	2	-437.291	3	-107.653	1	0	1	-.032	4	-.412	2
507	7	max	473.864	3	475.51	1	27.341	5	0	3	-.006	12	.366	3
508		min	-286.517	2	-438.386	3	-107.653	1	0	1	-.097	1	-.698	2
509	8	max	474.496	3	474.051	1	28.801	5	0	3	.009	5	.638	3
510		min	-285.675	2	-439.48	3	-107.653	1	0	1	-.164	1	-.991	1
511	9	max	491.887	3	41.668	2	71.407	5	0	9	.101	1	.746	3
512		min	-193.969	2	.439	15	-166.343	1	0	3	-.172	5	-1.13	1
513	10	max	492.519	3	40.209	2	72.867	5	0	9	0	12	.727	3
514		min	-193.126	2	-.004	5	-166.343	1	0	3	-.129	4	-1.15	2
515	11	max	493.15	3	38.75	2	74.327	5	0	9	-.006	12	.71	3
516		min	-192.284	2	-1.791	4	-166.343	1	0	3	-.106	4	-1.175	2
517	12	max	510.43	3	288.848	3	186.806	5	0	2	.161	1	.619	3
518		min	-109.299	5	-544.473	2	-103.534	1	0	3	-.291	5	-1.041	2
519	13	max	511.062	3	287.753	3	188.266	5	0	2	.096	1	.44	3
520		min	-108.906	5	-545.932	2	-103.534	1	0	3	-.175	5	-.702	2
521	14	max	511.694	3	286.659	3	189.726	5	0	2	.032	1	.262	3
522		min	-108.513	5	-547.391	2	-103.534	1	0	3	-.058	5	-.376	1
523	15	max	512.325	3	285.565	3	191.186	5	0	2	.061	5	.084	3
524		min	-108.12	5	-548.85	2	-103.534	1	0	3	-.032	1	-.052	1
525	16	max	512.957	3	284.47	3	192.646	5	0	2	.18	5	.318	2
526		min	-107.726	5	-550.309	2	-103.534	1	0	3	-.096	1	-.093	3
527	17	max	513.589	3	283.376	3	194.106	5	0	2	.3	5	.66	2
528		min	-107.333	5	-551.768	2	-103.534	1	0	3	-.161	1	-.269	3
529	18	max	26.537	5	549.686	2	-6.822	12	0	5	.268	5	.331	2
530		min	-212.863	1	-230.142	3	-144.597	4	0	2	-.231	1	-.132	3
531	19	max	26.931	5	548.227	2	-6.822	12	0	5	.197	5	.011	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-212.021	1	-231.237	3	-143.137	4	0	2	-.307	1	-.011	1
533	M5	max	462.917	1	1957.171	3	111.683	5	0	1	0	1	.029	2
534		min	20.271	12	-1408.3	1	0	1	0	4	-.301	4	0	3
535		max	463.759	1	1956.077	3	113.143	5	0	1	0	1	.902	1
536		min	20.692	12	-1409.759	1	0	1	0	4	-.232	4	-1.214	3
537		max	1495.667	3	1406.958	1	83.441	4	0	4	0	1	1.746	1
538		min	-1005.433	2	-1362.61	3	0	1	0	1	-.162	4	-2.391	3
539		max	1496.298	3	1405.499	1	84.901	4	0	4	0	1	.873	1
540		min	-1004.59	2	-1363.704	3	0	1	0	1	-.109	4	-1.545	3
541		max	1496.93	3	1404.04	1	86.361	4	0	4	0	1	.024	9
542		min	-1003.748	2	-1364.798	3	0	1	0	1	-.056	4	-.699	3
543		max	1497.562	3	1402.581	1	87.821	4	0	4	0	1	.149	3
544		min	-1002.906	2	-1365.893	3	0	1	0	1	-.002	5	-.894	2
545		max	1498.194	3	1401.122	1	89.281	4	0	4	.053	4	.997	3
546		min	-1002.063	2	-1366.987	3	0	1	0	1	0	1	-1.739	1
547		max	1498.826	3	1399.663	1	90.741	4	0	4	.109	4	1.845	3
548		min	-1001.221	2	-1368.081	3	0	1	0	1	0	1	-2.609	1
549		max	1528.562	3	139.322	2	238.981	4	0	1	0	1	2.126	3
550		min	-811.61	2	.445	15	0	1	0	1	-.262	4	-2.96	1
551		max	1529.194	3	137.863	2	240.442	4	0	1	0	1	2.059	3
552		min	-810.768	2	.005	15	0	1	0	1	-.114	4	-3.015	2
553		max	1529.826	3	136.403	2	241.902	4	0	1	.036	4	1.992	3
554		min	-809.925	2	-1.458	6	0	1	0	1	0	1	-3.1	2
555		max	1559.786	3	887.367	3	265.718	4	0	1	0	1	1.748	3
556		min	-620.332	2	-1627.212	2	0	1	0	4	-.426	4	-2.772	2
557		max	1560.417	3	886.273	3	267.178	4	0	1	0	1	1.198	3
558		min	-619.49	2	-1628.671	2	0	1	0	4	-.26	4	-1.761	2
559		max	1561.049	3	885.179	3	268.638	4	0	1	0	1	.648	3
560		min	-618.648	2	-1630.13	2	0	1	0	4	-.094	4	-.794	1
561		max	1561.681	3	884.084	3	270.098	4	0	1	.073	4	.262	2
562		min	-617.805	2	-1631.589	2	0	1	0	4	0	1	0	13
563		max	1562.313	3	882.99	3	271.558	4	0	1	.241	4	1.275	2
564		min	-616.963	2	-1633.048	2	0	1	0	4	0	1	-.449	3
565		max	1562.945	3	881.896	3	273.018	4	0	1	.41	4	2.289	2
566		min	-616.12	2	-1634.507	2	0	1	0	4	0	1	-.997	3
567		max	-21.091	12	1858.889	2	0	1	0	4	.429	4	1.173	2
568		min	-462.823	1	-800.047	3	-28.098	5	0	1	0	1	-.519	3
569		max	-20.67	12	1857.43	2	0	1	0	4	.413	4	.023	1
570		min	-461.98	1	-801.141	3	-26.638	5	0	1	0	1	-.022	3
571	M9	max	211.552	1	587.636	3	107.925	1	0	3	-.018	12	0	3
572		min	10.739	12	-413.278	1	6.375	12	0	4	-.304	1	-.014	2
573		max	212.395	1	586.542	3	107.925	1	0	3	-.014	12	.243	1
574		min	11.16	12	-414.737	1	6.375	12	0	4	-.237	1	-.365	3
575		max	471.337	3	481.346	1	107.653	1	0	1	-.01	12	.491	1
576		min	-289.887	2	-434.008	3	6.342	12	0	3	-.17	1	-.717	3
577		max	471.969	3	479.887	1	107.653	1	0	1	-.006	12	.193	1
578		min	-289.044	2	-435.103	3	6.342	12	0	3	-.103	1	-.447	3
579		max	472.601	3	478.428	1	107.653	1	0	1	-.002	12	-.005	15
580		min	-288.202	2	-436.197	3	6.342	12	0	3	-.055	4	-.177	3
581		max	473.232	3	476.969	1	107.653	1	0	1	.031	1	.094	3
582		min	-287.36	2	-437.291	3	6.342	12	0	3	-.021	5	-.412	2
583		max	473.864	3	475.51	1	107.653	1	0	1	.097	1	.366	3
584		min	-286.517	2	-438.386	3	6.342	12	0	3	.003	15	-.698	2
585		max	474.496	3	474.051	1	107.653	1	0	1	.164	1	.638	3
586		min	-285.675	2	-439.48	3	6.342	12	0	3	.01	12	-.991	1
587		max	491.887	3	41.668	2	166.343	1	0	3	-.006	12	.746	3
588		min	-193.969	2	.454	15	9.549	12	0	9	-.212	4	-1.13	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	492.519	3	40.209	2	166.343	1	0	3	.002	1	.727	3
590		min	-193.126	2	.013	15	9.549	12	0	9	-.127	4	-1.15	2
591	11	max	493.15	3	38.75	2	166.343	1	0	3	.105	1	.71	3
592		min	-192.284	2	-1.676	6	9.549	12	0	9	-.066	5	-1.175	2
593	12	max	510.43	3	288.848	3	229.476	4	0	3	-.009	12	.619	3
594		min	-106.691	10	-544.473	2	5.783	12	0	2	-.357	4	-1.041	2
595	13	max	511.062	3	287.753	3	230.936	4	0	3	-.005	12	.44	3
596		min	-105.989	10	-545.932	2	5.783	12	0	2	-.214	4	-.702	2
597	14	max	511.694	3	286.659	3	232.396	4	0	3	-.002	12	.262	3
598		min	-105.287	10	-547.391	2	5.783	12	0	2	-.07	4	-.376	1
599	15	max	512.325	3	285.565	3	233.857	4	0	3	.074	4	.084	3
600		min	-104.585	10	-548.85	2	5.783	12	0	2	.002	12	-.052	1
601	16	max	512.957	3	284.47	3	235.317	4	0	3	.22	4	.318	2
602		min	-103.883	10	-550.309	2	5.783	12	0	2	.005	12	-.093	3
603	17	max	513.589	3	283.376	3	236.777	4	0	3	.367	4	.66	2
604		min	-103.181	10	-551.768	2	5.783	12	0	2	.009	12	-.269	3
605	18	max	-10.961	12	549.686	2	121.909	1	0	2	.36	4	.331	2
606		min	-212.863	1	-230.142	3	-97.887	5	0	3	.013	12	-.132	3
607	19	max	-10.539	12	548.227	2	121.909	1	0	2	.317	4	.011	3
608		min	-212.021	1	-231.237	3	-96.427	5	0	3	.017	12	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.168	2	.009	3	1.154e-2	2	NC	1	NC	1
2			min	-1.035	4	-.032	3	-.005	2	-2.238e-3	3	NC	1	NC	1
3		2	max	0	1	.247	3	.052	1	1.294e-2	2	NC	5	NC	2
4			min	-1.035	4	-.01	9	-.031	5	-2.222e-3	3	861.292	3	4779.372	1
5		3	max	0	1	.473	3	.122	1	1.434e-2	2	NC	5	NC	3
6			min	-1.035	4	-.139	1	-.037	5	-2.207e-3	3	475.83	3	1995.947	1
7		4	max	0	1	.61	3	.181	1	1.575e-2	2	NC	5	NC	3
8			min	-1.035	4	-.211	1	-.027	5	-2.192e-3	3	374.043	3	1333.605	1
9		5	max	0	1	.642	3	.211	1	1.715e-2	2	NC	5	NC	3
10			min	-1.035	4	-.208	1	-.007	5	-2.177e-3	3	356.185	3	1142.745	1
11		6	max	0	1	.571	3	.203	1	1.855e-2	2	NC	5	NC	5
12			min	-1.035	4	-.135	1	.009	15	-2.162e-3	3	397.861	3	1189.131	1
13		7	max	0	1	.419	3	.159	1	1.995e-2	2	NC	5	NC	10
14			min	-1.035	4	-.016	9	.011	10	-2.146e-3	3	531.946	3	1523.189	1
15		8	max	0	1	.226	3	.092	1	2.135e-2	2	NC	1	NC	3
16			min	-1.035	4	.005	15	.001	10	-2.131e-3	3	930.688	3	2661.529	1
17		9	max	0	1	.296	2	.038	4	2.275e-2	2	NC	4	NC	1
18			min	-1.035	4	.009	15	-.008	10	-2.116e-3	3	1874.406	2	6252.889	4
19		10	max	0	1	.352	2	.028	3	2.415e-2	2	NC	3	NC	1
20			min	-1.035	4	-.029	3	-.019	2	-2.101e-3	3	1304.804	2	NC	1
21		11	max	0	12	.296	2	.03	3	2.275e-2	2	NC	4	NC	1
22			min	-1.035	4	.009	15	-.024	5	-2.116e-3	3	1874.406	2	NC	1
23		12	max	0	12	.226	3	.092	1	2.135e-2	2	NC	1	NC	3
24			min	-1.035	4	.005	15	-.023	5	-2.131e-3	3	930.688	3	2661.529	1
25		13	max	0	12	.419	3	.159	1	1.995e-2	2	NC	5	NC	5
26			min	-1.035	4	-.016	9	-.007	5	-2.146e-3	3	531.946	3	1523.189	1
27		14	max	0	12	.571	3	.203	1	1.855e-2	2	NC	5	NC	5
28			min	-1.035	4	-.135	1	.01	15	-2.162e-3	3	397.861	3	1189.131	1
29		15	max	0	12	.642	3	.211	1	1.715e-2	2	NC	5	NC	3
30			min	-1.035	4	-.208	1	.02	10	-2.177e-3	3	356.185	3	1142.745	1
31		16	max	0	12	.61	3	.181	1	1.575e-2	2	NC	5	NC	3
32			min	-1.035	4	-.211	1	.017	10	-2.192e-3	3	374.043	3	1333.605	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.473	3	.122	1	1.434e-2	2	NC	5	NC	3
34		min	-1.035	4	-.139	1	.011	10	-2.207e-3	3	475.83	3	1995.947	1
35	18	max	0	12	.247	3	.052	4	1.294e-2	2	NC	5	NC	2
36		min	-1.035	4	-.01	9	.003	10	-2.222e-3	3	861.292	3	4527.551	4
37	19	max	0	12	.168	2	.009	3	1.154e-2	2	NC	1	NC	1
38		min	-1.035	4	-.032	3	-.005	2	-2.238e-3	3	NC	1	NC	1
39	M14	1	max	0	.308	3	.008	3	6.696e-3	1	NC	1	NC	1
40		min	-.748	4	-.518	2	-.004	2	-4.668e-3	3	NC	1	NC	1
41	2	max	0	1	.609	3	.034	1	7.903e-3	1	NC	5	NC	2
42		min	-.748	4	-.846	1	-.046	5	-5.616e-3	3	723.801	1	5462.987	5
43	3	max	0	1	.868	3	.095	1	9.11e-3	1	NC	15	NC	3
44		min	-.748	4	-1.136	1	-.055	5	-6.564e-3	3	385.928	1	2563.613	1
45	4	max	0	1	1.055	3	.152	1	1.032e-2	1	9324.955	15	NC	3
46		min	-.748	4	-1.356	1	-.038	5	-7.512e-3	3	285.009	1	1592.049	1
47	5	max	0	1	1.155	3	.185	1	1.152e-2	1	8068.611	15	NC	3
48		min	-.748	4	-1.491	1	-.007	5	-8.46e-3	3	245.651	1	1310.324	1
49	6	max	0	1	1.168	3	.182	1	1.273e-2	1	7732.605	15	NC	3
50		min	-.748	4	-1.539	1	.016	10	-9.407e-3	3	234.134	1	1328.83	1
51	7	max	0	1	1.109	3	.145	1	1.394e-2	1	7999.255	15	NC	3
52		min	-.748	4	-1.512	1	.01	10	-1.036e-2	3	240.412	1	1671.074	1
53	8	max	0	1	1.004	3	.088	4	1.515e-2	1	8740.244	15	NC	3
54		min	-.748	4	-1.436	1	.002	10	-1.13e-2	3	260.204	1	2703.512	4
55	9	max	0	1	.898	3	.058	4	1.635e-2	1	9734.342	15	NC	1
56		min	-.748	4	-1.351	1	-.007	10	-1.225e-2	3	286.901	1	4068.488	4
57	10	max	0	1	.847	3	.025	3	1.756e-2	1	NC	15	NC	1
58		min	-.748	4	-1.311	2	-.017	2	-1.32e-2	3	302.386	1	NC	1
59	11	max	0	12	.898	3	.026	3	1.635e-2	1	9734.306	15	NC	1
60		min	-.748	4	-1.351	1	-.045	5	-1.225e-2	3	286.901	1	5580.726	5
61	12	max	0	12	1.004	3	.085	1	1.515e-2	1	8740.14	15	NC	3
62		min	-.748	4	-1.436	1	-.051	5	-1.13e-2	3	260.204	1	2874.906	1
63	13	max	0	12	1.109	3	.145	1	1.394e-2	1	7999.085	15	NC	3
64		min	-.748	4	-1.512	1	-.033	5	-1.036e-2	3	240.412	1	1671.074	1
65	14	max	0	12	1.168	3	.182	1	1.273e-2	1	7732.367	15	NC	3
66		min	-.748	4	-1.539	1	0	15	-9.407e-3	3	234.134	1	1328.83	1
67	15	max	0	12	1.155	3	.185	1	1.152e-2	1	8068.287	15	NC	3
68		min	-.748	4	-1.491	1	.018	10	-8.46e-3	3	245.651	1	1310.324	1
69	16	max	0	12	1.055	3	.152	1	1.032e-2	1	9324.488	15	NC	3
70		min	-.748	4	-1.356	1	.014	10	-7.512e-3	3	285.009	1	1592.049	1
71	17	max	0	12	.868	3	.095	1	9.11e-3	1	NC	15	NC	3
72		min	-.748	4	-1.136	1	.008	10	-6.564e-3	3	385.928	1	2551.469	4
73	18	max	0	12	.609	3	.061	4	7.903e-3	1	NC	5	NC	2
74		min	-.748	4	-.846	1	0	10	-5.616e-3	3	723.801	1	3898.992	4
75	19	max	0	12	.308	3	.008	3	6.696e-3	1	NC	1	NC	1
76		min	-.748	4	-.518	2	-.004	2	-4.668e-3	3	NC	1	NC	1
77	M15	1	max	0	.315	3	.008	3	3.966e-3	3	NC	1	NC	1
78		min	-.592	4	-.517	2	-.004	2	-6.954e-3	2	NC	1	NC	1
79	2	max	0	12	.518	3	.035	1	4.773e-3	3	NC	5	NC	2
80		min	-.592	4	-.91	2	-.061	5	-8.21e-3	2	610.375	2	4055.39	5
81	3	max	0	12	.698	3	.096	1	5.581e-3	3	NC	15	NC	3
82		min	-.592	4	-1.25	2	-.074	5	-9.467e-3	2	327.262	2	2550.841	1
83	4	max	0	12	.838	3	.153	1	6.388e-3	3	9340.673	15	NC	3
84		min	-.592	4	-1.501	2	-.054	5	-1.072e-2	2	243.961	2	1585.994	1
85	5	max	0	12	.928	3	.185	1	7.195e-3	3	8083.857	15	NC	3
86		min	-.592	4	-1.642	2	-.014	5	-1.198e-2	2	213.229	2	1305.906	1
87	6	max	0	12	.969	3	.183	1	8.002e-3	3	7749.439	15	NC	3
88		min	-.592	4	-1.675	2	.016	10	-1.324e-2	2	207.314	2	1324.299	1
89	7	max	0	12	.964	3	.146	1	8.809e-3	3	8019.811	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-592	4	-1.614	2	.011	10	-1.449e-2	2	218.802	2	1664.268	1
91		8	max	0	12	.93	3	.104	4	9.616e-3	3	8767.111	15	NC	3
92			min	-592	4	-1.495	2	.002	10	-1.575e-2	2	245.446	2	2297.445	4
93		9	max	0	12	.888	3	.071	4	1.042e-2	3	9769.52	15	NC	1
94			min	-592	4	-1.37	2	-.006	10	-1.701e-2	2	281.396	2	3335.125	4
95		10	max	0	1	.865	3	.023	3	1.123e-2	3	NC	15	NC	1
96			min	-592	4	-1.309	2	-.016	2	-1.826e-2	2	302.887	2	NC	1
97		11	max	0	1	.888	3	.025	3	1.042e-2	3	9769.489	15	NC	1
98			min	-592	4	-1.37	2	-.058	5	-1.701e-2	2	281.396	2	4278.464	5
99		12	max	0	1	.93	3	.086	1	9.616e-3	3	8767.032	15	NC	3
100			min	-592	4	-1.495	2	-.067	5	-1.575e-2	2	245.446	2	2856.274	1
101		13	max	0	1	.964	3	.146	1	8.809e-3	3	8019.689	15	NC	3
102			min	-592	4	-1.614	2	-.044	5	-1.449e-2	2	218.802	2	1664.268	1
103		14	max	0	1	.969	3	.183	1	8.002e-3	3	7749.272	15	NC	3
104			min	-592	4	-1.675	2	-.003	5	-1.324e-2	2	207.314	2	1324.299	1
105		15	max	0	1	.928	3	.185	1	7.195e-3	3	8083.632	15	NC	3
106			min	-592	4	-1.642	2	.017	12	-1.198e-2	2	213.229	2	1305.906	1
107		16	max	0	1	.838	3	.153	1	6.388e-3	3	9340.351	15	NC	3
108			min	-592	4	-1.501	2	.014	12	-1.072e-2	2	243.961	2	1585.994	1
109		17	max	0	1	.698	3	.113	4	5.581e-3	3	NC	15	NC	3
110			min	-592	4	-1.25	2	.008	10	-9.467e-3	2	327.262	2	2110.068	4
111		18	max	0	1	.518	3	.076	4	4.773e-3	3	NC	5	NC	2
112			min	-592	4	-.91	2	.001	10	-8.21e-3	2	610.375	2	3123.155	4
113		19	max	0	1	.315	3	.008	3	3.966e-3	3	NC	1	NC	1
114			min	-592	4	-.517	2	-.004	2	-6.954e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.158	1	.006	3	7.185e-3	3	NC	1	NC	1
116			min	-149	4	-1.106	3	-.004	2	-1.022e-2	1	NC	1	NC	1
117		2	max	0	12	.003	13	.051	1	8.284e-3	3	NC	5	NC	2
118			min	-149	4	-.088	2	-.044	5	-1.133e-2	1	1011.221	2	4844.507	1
119		3	max	0	12	.047	3	.121	1	9.383e-3	3	NC	5	NC	3
120			min	-149	4	-.276	2	-.055	5	-1.243e-2	1	564.054	2	2010.591	1
121		4	max	0	12	.079	3	.18	1	1.048e-2	3	NC	5	NC	3
122			min	-149	4	-.382	2	-.042	5	-1.354e-2	1	451.449	2	1338.868	1
123		5	max	0	12	.07	3	.211	1	1.158e-2	3	NC	5	NC	3
124			min	-149	4	-.391	2	-.015	5	-1.465e-2	1	443.858	2	1144.245	1
125		6	max	0	12	.023	3	.203	1	1.268e-2	3	NC	5	NC	3
126			min	-149	4	-.306	2	.01	15	-1.576e-2	1	526.8	2	1187.279	1
127		7	max	0	12	.002	13	.16	1	1.378e-2	3	NC	5	NC	3
128			min	-149	4	-.147	2	.013	10	-1.687e-2	1	808.449	2	1513.987	1
129		8	max	0	12	.093	1	.093	1	1.488e-2	3	NC	4	NC	3
130			min	-149	4	-.145	3	.004	10	-1.798e-2	1	2329.357	2	2616.615	1
131		9	max	0	12	.252	1	.048	4	1.598e-2	3	NC	4	NC	2
132			min	-149	4	-.223	3	-.005	10	-1.909e-2	1	2054.577	3	4951.718	4
133		10	max	0	1	.323	1	.02	3	1.708e-2	3	NC	5	NC	1
134			min	-149	4	-.258	3	-.015	2	-2.02e-2	1	1453.863	1	NC	1
135		11	max	0	1	.252	1	.027	1	1.598e-2	3	NC	4	NC	2
136			min	-149	4	-.223	3	-.034	5	-1.909e-2	1	2054.577	3	7327.546	5
137		12	max	0	1	.093	1	.093	1	1.488e-2	3	NC	4	NC	3
138			min	-148	4	-.145	3	-.035	5	-1.798e-2	1	2329.357	2	2616.615	1
139		13	max	0	1	.002	13	.16	1	1.378e-2	3	NC	5	NC	3
140			min	-148	4	-.147	2	-.015	5	-1.687e-2	1	808.449	2	1513.987	1
141		14	max	0	1	.023	3	.203	1	1.268e-2	3	NC	5	NC	3
142			min	-148	4	-.306	2	.01	15	-1.576e-2	1	526.8	2	1187.279	1
143		15	max	0	1	.07	3	.211	1	1.158e-2	3	NC	5	NC	3
144			min	-148	4	-.391	2	.018	12	-1.465e-2	1	443.858	2	1144.245	1
145		16	max	0	1	.079	3	.18	1	1.048e-2	3	NC	5	NC	3
146			min	-148	4	-.382	2	.015	12	-1.354e-2	1	451.449	2	1338.868	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.047	3	.121	1	9.383e-3	3	NC	5	NC	3
148			min	-.148	4	-.276	2	.011	12	-1.243e-2	1	564.054	2	2010.591	1
149		18	max	.001	1	.002	13	.065	4	8.284e-3	3	NC	5	NC	2
150			min	-.148	4	-.088	2	.003	10	-1.133e-2	1	1011.221	2	3644.517	4
151		19	max	.001	1	.158	1	.006	3	7.185e-3	3	NC	1	NC	1
152			min	-.148	4	-.106	3	-.004	2	-1.022e-2	1	NC	1	NC	1
153	M2	1	max	.007	1	.008	2	.014	1	2.699e-3	5	NC	1	NC	2
154			min	-.008	3	-.014	3	-.964	4	-3.332e-4	1	9391.226	2	80.364	4
155		2	max	.007	1	.007	2	.013	1	2.762e-3	5	NC	1	NC	2
156			min	-.008	3	-.013	3	-.886	4	-3.163e-4	1	NC	1	87.484	4
157		3	max	.007	1	.005	2	.012	1	2.824e-3	5	NC	1	NC	2
158			min	-.007	3	-.013	3	-.808	4	-2.994e-4	1	NC	1	95.936	4
159		4	max	.006	1	.004	2	.011	1	2.886e-3	5	NC	1	NC	2
160			min	-.007	3	-.013	3	-.731	4	-2.824e-4	1	NC	1	106.065	4
161		5	max	.006	1	.003	2	.01	1	2.948e-3	5	NC	1	NC	2
162			min	-.006	3	-.012	3	-.655	4	-2.655e-4	1	NC	1	118.345	4
163		6	max	.005	1	.002	2	.008	1	3.01e-3	5	NC	1	NC	2
164			min	-.006	3	-.012	3	-.581	4	-2.486e-4	1	NC	1	133.425	4
165		7	max	.005	1	0	2	.007	1	3.072e-3	5	NC	1	NC	1
166			min	-.006	3	-.012	3	-.509	4	-2.317e-4	1	NC	1	152.232	4
167		8	max	.005	1	0	2	.006	1	3.134e-3	5	NC	1	NC	1
168			min	-.005	3	-.011	3	-.44	4	-2.147e-4	1	NC	1	176.113	4
169		9	max	.004	1	0	15	.005	1	3.198e-3	4	NC	1	NC	1
170			min	-.005	3	-.011	3	-.374	4	-1.978e-4	1	NC	1	207.094	4
171		10	max	.004	1	-.001	15	.005	1	3.266e-3	4	NC	1	NC	1
172			min	-.004	3	-.01	3	-.312	4	-1.809e-4	1	NC	1	248.338	4
173		11	max	.003	1	-.001	15	.004	1	3.334e-3	4	NC	1	NC	1
174			min	-.004	3	-.009	3	-.254	4	-1.64e-4	1	NC	1	305.014	4
175		12	max	.003	1	-.001	15	.003	1	3.402e-3	4	NC	1	NC	1
176			min	-.003	3	-.008	3	-.201	4	-1.47e-4	1	NC	1	386.048	4
177		13	max	.002	1	-.001	15	.002	1	3.47e-3	4	NC	1	NC	1
178			min	-.003	3	-.008	3	-.153	4	-1.301e-4	1	NC	1	507.975	4
179		14	max	.002	1	-.001	15	.002	1	3.538e-3	4	NC	1	NC	1
180			min	-.002	3	-.007	3	-.11	4	-1.132e-4	1	NC	1	704.35	4
181		15	max	.002	1	-.001	15	.001	1	3.606e-3	4	NC	1	NC	1
182			min	-.002	3	-.006	3	-.074	4	-9.628e-5	1	NC	1	1052.271	4
183		16	max	.001	1	0	15	0	1	3.674e-3	4	NC	1	NC	1
184			min	-.001	3	-.004	6	-.044	4	-7.935e-5	1	NC	1	1763.875	4
185		17	max	0	1	0	15	0	1	3.742e-3	4	NC	1	NC	1
186			min	0	3	-.003	6	-.021	4	-6.243e-5	1	NC	1	3620.01	4
187		18	max	0	1	0	15	0	1	3.81e-3	4	NC	1	NC	1
188			min	0	3	-.002	6	-.007	4	-4.551e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	3.878e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-2.859e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	5.418e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-8.42e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.021	4	4.179e-5	1	NC	1	NC	1
194			min	0	2	-.003	6	0	1	-2.535e-5	5	NC	1	NC	1
195		3	max	0	3	-.001	15	.04	4	8.114e-4	4	NC	1	NC	1
196			min	0	2	-.006	6	0	1	4.318e-6	12	NC	1	7954.73	5
197		4	max	.001	3	-.002	15	.058	4	1.638e-3	4	NC	1	NC	1
198			min	0	2	-.009	6	0	1	6.301e-6	12	NC	1	6002.957	5
199		5	max	.002	3	-.003	15	.074	4	2.465e-3	4	NC	1	NC	1
200			min	-.001	2	-.012	6	0	1	8.284e-6	12	8520.357	6	5157.93	5
201		6	max	.002	3	-.003	15	.089	4	3.292e-3	4	NC	5	NC	1
202			min	-.002	2	-.015	6	0	1	1.027e-5	12	6904.454	6	4794.105	5
203		7	max	.002	3	-.004	15	.103	4	4.118e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.017	6	0	1	1.225e-5	12	5931.236	6	4719.521	5
205		8	max	.003	3	-.004	15	.116	4	4.945e-3	4	NC	5	NC	1
206			min	-.002	2	-.019	6	0	12	1.423e-5	12	5331.006	6	4876.016	5
207		9	max	.003	3	-.005	15	.128	4	5.772e-3	4	NC	5	NC	1
208			min	-.003	2	-.021	6	0	12	1.622e-5	12	4976.818	6	5272.409	5
209		10	max	.004	3	-.005	15	.139	4	6.599e-3	4	NC	5	NC	1
210			min	-.003	2	-.021	6	0	12	1.82e-5	12	4807.129	6	5978.494	5
211		11	max	.004	3	-.005	15	.15	4	7.425e-3	4	NC	5	NC	1
212			min	-.003	2	-.021	6	0	12	2.018e-5	12	4796.991	6	7159.244	5
213		12	max	.004	3	-.005	15	.161	4	8.252e-3	4	NC	5	NC	1
214			min	-.003	2	-.021	6	0	12	2.217e-5	12	4948.492	6	9186.08	5
215		13	max	.005	3	-.004	15	.171	4	9.079e-3	4	NC	5	NC	1
216			min	-.004	2	-.019	6	0	12	2.415e-5	12	5292.785	6	NC	1
217		14	max	.005	3	-.004	15	.182	4	9.905e-3	4	NC	5	NC	1
218			min	-.004	2	-.017	6	0	12	2.613e-5	12	5906.173	6	NC	1
219		15	max	.006	3	-.003	15	.193	4	1.073e-2	4	NC	3	NC	1
220			min	-.004	2	-.015	6	0	12	2.812e-5	12	6957.519	6	NC	1
221		16	max	.006	3	-.002	15	.204	4	1.156e-2	4	NC	1	NC	1
222			min	-.005	2	-.012	6	0	12	3.01e-5	12	8855.603	6	NC	1
223		17	max	.006	3	-.002	15	.217	4	1.239e-2	4	NC	1	NC	1
224			min	-.005	2	-.008	1	0	12	3.209e-5	12	NC	1	NC	1
225		18	max	.007	3	0	15	.231	4	1.321e-2	4	NC	1	NC	2
226			min	-.005	2	-.005	1	0	12	3.407e-5	12	NC	1	9281.402	1
227		19	max	.007	3	0	5	.247	4	1.404e-2	4	NC	1	NC	2
228			min	-.006	2	-.002	1	0	12	3.605e-5	12	NC	1	7743.392	1
229	M4	1	max	.003	1	.005	2	0	12	2.337e-4	1	NC	1	NC	3
230			min	0	5	-.007	3	-.247	4	-6.414e-4	5	NC	1	100.619	4
231		2	max	.002	1	.005	2	0	12	2.337e-4	1	NC	1	NC	3
232			min	0	5	-.007	3	-.227	4	-6.414e-4	5	NC	1	109.447	4
233		3	max	.002	1	.005	2	0	12	2.337e-4	1	NC	1	NC	3
234			min	0	5	-.007	3	-.207	4	-6.414e-4	5	NC	1	119.95	4
235		4	max	.002	1	.004	2	0	12	2.337e-4	1	NC	1	NC	3
236			min	0	5	-.006	3	-.187	4	-6.414e-4	5	NC	1	132.565	4
237		5	max	.002	1	.004	2	0	12	2.337e-4	1	NC	1	NC	3
238			min	0	5	-.006	3	-.168	4	-6.414e-4	5	NC	1	147.885	4
239		6	max	.002	1	.004	2	0	12	2.337e-4	1	NC	1	NC	3
240			min	0	5	-.005	3	-.149	4	-6.414e-4	5	NC	1	166.729	4
241		7	max	.002	1	.004	2	0	12	2.337e-4	1	NC	1	NC	3
242			min	0	5	-.005	3	-.13	4	-6.414e-4	5	NC	1	190.265	4
243		8	max	.002	1	.003	2	0	12	2.337e-4	1	NC	1	NC	3
244			min	0	5	-.005	3	-.113	4	-6.414e-4	5	NC	1	220.191	4
245		9	max	.001	1	.003	2	0	12	2.337e-4	1	NC	1	NC	2
246			min	0	5	-.004	3	-.096	4	-6.414e-4	5	NC	1	259.067	4
247		10	max	.001	1	.003	2	0	12	2.337e-4	1	NC	1	NC	2
248			min	0	5	-.004	3	-.08	4	-6.414e-4	5	NC	1	310.894	4
249		11	max	.001	1	.002	2	0	12	2.337e-4	1	NC	1	NC	2
250			min	0	5	-.003	3	-.065	4	-6.414e-4	5	NC	1	382.217	4
251		12	max	.001	1	.002	2	0	12	2.337e-4	1	NC	1	NC	2
252			min	0	5	-.003	3	-.051	4	-6.414e-4	5	NC	1	484.357	4
253		13	max	0	1	.002	2	0	12	2.337e-4	1	NC	1	NC	1
254			min	0	5	-.002	3	-.039	4	-6.414e-4	5	NC	1	638.328	4
255		14	max	0	1	.001	2	0	12	2.337e-4	1	NC	1	NC	1
256			min	0	5	-.002	3	-.028	4	-6.414e-4	5	NC	1	886.881	4
257		15	max	0	1	.001	2	0	12	2.337e-4	1	NC	1	NC	1
258			min	0	5	-.002	3	-.019	4	-6.414e-4	5	NC	1	1328.564	4
259		16	max	0	1	0	2	0	12	2.337e-4	1	NC	1	NC	1
260			min	0	5	-.001	3	-.011	4	-6.414e-4	5	NC	1	2235.791	4



Company : Schletter, Inc.  
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Job Number :  
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Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	2.337e-4	1	NC	1	NC	1
262			min	0	5	0	3	-0.005	4	-6.414e-4	5	NC	1	4618.582	4
263		18	max	0	1	0	2	0	12	2.337e-4	1	NC	1	NC	1
264			min	0	5	0	3	-0.002	4	-6.414e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.337e-4	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-6.414e-4	5	NC	1	NC	1
267	M6	1	max	.023	1	.031	2	0	1	2.864e-3	4	NC	3	NC	1
268			min	-.027	3	-.043	3	-.975	4	0	1	2474.427	2	79.483	4
269		2	max	.022	1	.028	2	0	1	2.923e-3	4	NC	3	NC	1
270			min	-.025	3	-.041	3	-.896	4	0	1	2732.751	2	86.525	4
271		3	max	.021	1	.025	2	0	1	2.983e-3	4	NC	3	NC	1
272			min	-.024	3	-.038	3	-.817	4	0	1	3048.271	2	94.885	4
273		4	max	.019	1	.023	2	0	1	3.042e-3	4	NC	3	NC	1
274			min	-.022	3	-.036	3	-.739	4	0	1	3438.534	2	104.904	4
275		5	max	.018	1	.02	2	0	1	3.102e-3	4	NC	3	NC	1
276			min	-.021	3	-.034	3	-.662	4	0	1	3928.631	2	117.049	4
277		6	max	.017	1	.017	2	0	1	3.161e-3	4	NC	3	NC	1
278			min	-.019	3	-.032	3	-.587	4	0	1	4555.479	2	131.966	4
279		7	max	.016	1	.014	2	0	1	3.221e-3	4	NC	1	NC	1
280			min	-.018	3	-.029	3	-.515	4	0	1	5375.272	2	150.568	4
281		8	max	.014	1	.012	2	0	1	3.28e-3	4	NC	1	NC	1
282			min	-.016	3	-.027	3	-.445	4	0	1	6477.121	2	174.188	4
283		9	max	.013	1	.01	2	0	1	3.34e-3	4	NC	1	NC	1
284			min	-.015	3	-.025	3	-.378	4	0	1	8009.668	2	204.83	4
285		10	max	.012	1	.008	2	0	1	3.399e-3	4	NC	1	NC	1
286			min	-.013	3	-.022	3	-.315	4	0	1	NC	1	245.624	4
287		11	max	.01	1	.006	2	0	1	3.459e-3	4	NC	1	NC	1
288			min	-.012	3	-.02	3	-.257	4	0	1	NC	1	301.68	4
289		12	max	.009	1	.004	2	0	1	3.518e-3	4	NC	1	NC	1
290			min	-.01	3	-.018	3	-.203	4	0	1	NC	1	381.826	4
291		13	max	.008	1	.003	2	0	1	3.578e-3	4	NC	1	NC	1
292			min	-.009	3	-.015	3	-.154	4	0	1	NC	1	502.412	4
293		14	max	.006	1	.001	2	0	1	3.637e-3	4	NC	1	NC	1
294			min	-.007	3	-.013	3	-.111	4	0	1	NC	1	696.619	4
295		15	max	.005	1	0	2	0	1	3.697e-3	4	NC	1	NC	1
296			min	-.006	3	-.01	3	-.074	4	0	1	NC	1	1040.675	4
297		16	max	.004	1	0	2	0	1	3.756e-3	4	NC	1	NC	1
298			min	-.004	3	-.008	3	-.044	4	0	1	NC	1	1744.301	4
299		17	max	.003	1	0	2	0	1	3.816e-3	4	NC	1	NC	1
300			min	-.003	3	-.005	3	-.022	4	0	1	NC	1	3579.28	4
301		18	max	.001	1	0	2	0	1	3.875e-3	4	NC	1	NC	1
302			min	-.001	3	-.003	3	-.007	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	3.935e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-8.549e-4	4	NC	1	NC	1
307		2	max	.001	3	0	15	.021	4	0	1	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	-5.615e-5	4	NC	1	NC	1
309		3	max	.002	3	-.001	15	.041	4	7.426e-4	4	NC	1	NC	1
310			min	-.002	2	-.007	3	0	1	0	1	NC	1	7071.216	4
311		4	max	.004	3	-.002	15	.059	4	1.541e-3	4	NC	1	NC	1
312			min	-.004	2	-.01	3	0	1	0	1	NC	1	5282.925	4
313		5	max	.005	3	-.003	15	.075	4	2.34e-3	4	NC	1	NC	1
314			min	-.005	2	-.013	3	0	1	0	1	8575.985	4	4486.358	4
315		6	max	.006	3	-.004	15	.09	4	3.139e-3	4	NC	1	NC	1
316			min	-.006	2	-.015	3	0	1	0	1	6945.504	4	4112.418	4
317		7	max	.007	3	-.004	15	.104	4	3.937e-3	4	NC	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.007	2	-.018	4	0	1	0	1	5963.694	4	3981.329	4
319	8	max	.009	3	-.005	15	.117	4	4.736e-3	4	NC	5	NC	1
320		min	-.008	2	-.02	4	0	1	0	1	5358.103	4	4029.738	4
321	9	max	.01	3	-.005	15	.129	4	5.535e-3	4	NC	5	NC	1
322		min	-.01	2	-.021	4	0	1	0	1	5000.5	4	4246.099	4
323	10	max	.011	3	-.005	15	.14	4	6.334e-3	4	NC	5	NC	1
324		min	-.011	2	-.022	4	0	1	0	1	4828.695	4	4655.653	4
325	11	max	.012	3	-.005	15	.15	4	7.132e-3	4	NC	5	NC	1
326		min	-.012	2	-.022	4	0	1	0	1	4817.411	4	5327.218	4
327	12	max	.014	3	-.005	15	.16	4	7.931e-3	4	NC	5	NC	1
328		min	-.013	2	-.021	4	0	1	0	1	4968.599	4	6404.114	4
329	13	max	.015	3	-.005	15	.17	4	8.73e-3	4	NC	5	NC	1
330		min	-.014	2	-.02	4	0	1	0	1	5313.43	4	8188.28	4
331	14	max	.016	3	-.004	15	.179	4	9.528e-3	4	NC	5	NC	1
332		min	-.015	2	-.018	4	0	1	0	1	5928.411	4	NC	1
333	15	max	.017	3	-.004	15	.189	4	1.033e-2	4	NC	1	NC	1
334		min	-.017	2	-.016	3	0	1	0	1	6982.947	4	NC	1
335	16	max	.019	3	-.003	15	.199	4	1.113e-2	4	NC	1	NC	1
336		min	-.018	2	-.014	3	0	1	0	1	8887.199	4	NC	1
337	17	max	.02	3	-.002	15	.21	4	1.192e-2	4	NC	1	NC	1
338		min	-.019	2	-.011	3	0	1	0	1	NC	1	NC	1
339	18	max	.021	3	-.001	15	.222	4	1.272e-2	4	NC	1	NC	1
340		min	-.02	2	-.008	3	0	1	0	1	NC	1	NC	1
341	19	max	.022	3	0	10	.236	4	1.352e-2	4	NC	1	NC	1
342		min	-.021	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.021	2	0	0	1	NC	1	NC	1
344		min	0	3	-.023	3	-.236	4	-8.699e-4	4	NC	1	105.252	4
345	2	max	.006	1	.02	2	0	1	0	1	NC	1	NC	1
346		min	0	3	-.022	3	-.217	4	-8.699e-4	4	NC	1	114.505	4
347	3	max	.006	1	.018	2	0	1	0	1	NC	1	NC	1
348		min	0	3	-.02	3	-.198	4	-8.699e-4	4	NC	1	125.514	4
349	4	max	.006	1	.017	2	0	1	0	1	NC	1	NC	1
350		min	0	3	-.019	3	-.179	4	-8.699e-4	4	NC	1	138.736	4
351	5	max	.005	1	.016	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.018	3	-.16	4	-8.699e-4	4	NC	1	154.79	4
353	6	max	.005	1	.015	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.017	3	-.142	4	-8.699e-4	4	NC	1	174.538	4
355	7	max	.004	1	.014	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.015	3	-.125	4	-8.699e-4	4	NC	1	199.201	4
357	8	max	.004	1	.013	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.014	3	-.108	4	-8.699e-4	4	NC	1	230.561	4
359	9	max	.004	1	.011	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.013	3	-.091	4	-8.699e-4	4	NC	1	271.3	4
361	10	max	.003	1	.01	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.011	3	-.076	4	-8.699e-4	4	NC	1	325.611	4
363	11	max	.003	1	.009	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.01	3	-.062	4	-8.699e-4	4	NC	1	400.354	4
365	12	max	.003	1	.008	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.009	3	-.049	4	-8.699e-4	4	NC	1	507.394	4
367	13	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.008	3	-.037	4	-8.699e-4	4	NC	1	668.757	4
369	14	max	.002	1	.006	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.006	3	-.027	4	-8.699e-4	4	NC	1	929.253	4
371	15	max	.001	1	.005	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.005	3	-.018	4	-8.699e-4	4	NC	1	1392.179	4
373	16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.004	3	-.011	4	-8.699e-4	4	NC	1	2343.1	4





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	-.005	4	-8.699e-4	4	NC	1	4840.86	4
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.002	4	-8.699e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-8.699e-4	4	NC	1	NC	1
381	M10	1	max	.007	1	.008	2	0	12	2.857e-3	4	NC	1	NC	2
382			min	-.008	3	-.014	3	-.973	4	2.045e-5	12	9391.226	2	79.65	4
383		2	max	.007	1	.007	2	0	12	2.915e-3	4	NC	1	NC	2
384			min	-.008	3	-.013	3	-.894	4	1.941e-5	12	NC	1	86.707	4
385		3	max	.007	1	.005	2	0	12	2.972e-3	4	NC	1	NC	2
386			min	-.007	3	-.013	3	-.815	4	1.838e-5	12	NC	1	95.084	4
387		4	max	.006	1	.004	2	0	12	3.03e-3	4	NC	1	NC	2
388			min	-.007	3	-.013	3	-.737	4	1.734e-5	12	NC	1	105.126	4
389		5	max	.006	1	.003	2	0	12	3.088e-3	4	NC	1	NC	2
390			min	-.006	3	-.012	3	-.661	4	1.63e-5	12	NC	1	117.298	4
391		6	max	.005	1	.002	2	0	12	3.145e-3	4	NC	1	NC	2
392			min	-.006	3	-.012	3	-.586	4	1.527e-5	12	NC	1	132.248	4
393		7	max	.005	1	0	2	0	12	3.203e-3	4	NC	1	NC	1
394			min	-.006	3	-.012	3	-.514	4	1.423e-5	12	NC	1	150.892	4
395		8	max	.005	1	0	2	0	12	3.261e-3	4	NC	1	NC	1
396			min	-.005	3	-.011	3	-.444	4	1.32e-5	12	NC	1	174.566	4
397		9	max	.004	1	-.001	2	0	12	3.318e-3	4	NC	1	NC	1
398			min	-.005	3	-.011	3	-.377	4	1.216e-5	12	NC	1	205.28	4
399		10	max	.004	1	-.002	2	0	12	3.376e-3	4	NC	1	NC	1
400			min	-.004	3	-.01	3	-.315	4	1.113e-5	12	NC	1	246.17	4
401		11	max	.003	1	-.002	15	0	12	3.433e-3	4	NC	1	NC	1
402			min	-.004	3	-.009	3	-.256	4	1.009e-5	12	NC	1	302.362	4
403		12	max	.003	1	-.002	15	0	12	3.491e-3	4	NC	1	NC	1
404			min	-.003	3	-.008	3	-.202	4	9.057e-6	12	NC	1	382.706	4
405		13	max	.002	1	-.002	15	0	12	3.549e-3	4	NC	1	NC	1
406			min	-.003	3	-.008	4	-.154	4	8.021e-6	12	NC	1	503.602	4
407		14	max	.002	1	-.002	15	0	12	3.606e-3	4	NC	1	NC	1
408			min	-.002	3	-.007	4	-.111	4	6.986e-6	12	NC	1	698.331	4
409		15	max	.002	1	-.002	15	0	12	3.664e-3	4	NC	1	NC	1
410			min	-.002	3	-.006	4	-.074	4	5.95e-6	12	NC	1	1043.366	4
411		16	max	.001	1	-.001	15	0	12	3.722e-3	4	NC	1	NC	1
412			min	-.001	3	-.005	4	-.044	4	4.915e-6	12	NC	1	1749.167	4
413		17	max	0	1	0	15	0	12	3.779e-3	4	NC	1	NC	1
414			min	0	3	-.004	4	-.022	4	3.879e-6	12	NC	1	3590.591	4
415		18	max	0	1	0	15	0	12	3.837e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.007	4	2.844e-6	12	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.895e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	1.809e-6	12	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-3.507e-7	12	NC	1	NC	1
420			min	0	1	0	1	0	1	-8.452e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.021	4	-2.334e-6	12	NC	1	NC	1
422			min	0	2	-.003	4	0	12	-4.179e-5	1	NC	1	NC	1
423		3	max	0	3	-.002	15	.04	4	7.668e-4	5	NC	1	NC	1
424			min	0	2	-.006	4	0	12	-7.816e-5	1	NC	1	7454.887	4
425		4	max	.001	3	-.002	15	.058	4	1.568e-3	4	NC	1	NC	1
426			min	0	2	-.01	4	0	12	-1.145e-4	1	NC	1	5599.909	4
427		5	max	.002	3	-.003	15	.074	4	2.372e-3	4	NC	1	NC	1
428			min	-.001	2	-.013	4	0	12	-1.509e-4	1	8243.393	4	4785.446	4
429		6	max	.002	3	-.004	15	.089	4	3.177e-3	4	NC	5	NC	1
430			min	-.002	2	-.016	4	0	12	-1.873e-4	1	6699.357	4	4418.855	4
431		7	max	.002	3	-.004	15	.103	4	3.981e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432		min	-.002	2	-.018	4	0	10	-2.237e-4	1	5768.609	4	4315.447	4
433	8	max	.003	3	-.005	15	.116	4	4.786e-3	4	NC	5	NC	1
434		min	-.002	2	-.02	4	0	1	-2.6e-4	1	5194.924	4	4414.291	4
435	9	max	.003	3	-.005	15	.128	4	5.59e-3	4	NC	5	NC	1
436		min	-.003	2	-.022	4	0	1	-2.964e-4	1	4857.653	4	4712.604	4
437	10	max	.004	3	-.006	15	.139	4	6.394e-3	4	NC	5	NC	1
438		min	-.003	2	-.022	4	-.001	1	-3.328e-4	1	4698.434	4	5254.193	4
439	11	max	.004	3	-.006	15	.149	4	7.199e-3	4	NC	5	NC	1
440		min	-.003	2	-.023	4	-.002	1	-3.691e-4	1	4693.931	4	6146.438	4
441	12	max	.004	3	-.005	15	.159	4	8.003e-3	4	NC	5	NC	1
442		min	-.003	2	-.022	4	-.002	1	-4.055e-4	1	4846.893	4	7619.194	4
443	13	max	.005	3	-.005	15	.169	4	8.808e-3	4	NC	5	NC	1
444		min	-.004	2	-.021	4	-.003	1	-4.419e-4	1	5188.366	4	NC	1
445	14	max	.005	3	-.005	15	.179	4	9.612e-3	4	NC	5	NC	1
446		min	-.004	2	-.019	4	-.004	1	-4.783e-4	1	5793.604	4	NC	1
447	15	max	.006	3	-.004	15	.189	4	1.042e-2	4	NC	3	NC	1
448		min	-.004	2	-.016	4	-.006	1	-5.146e-4	1	6828.716	4	NC	1
449	16	max	.006	3	-.003	15	.2	4	1.122e-2	4	NC	1	NC	1
450		min	-.005	2	-.013	4	-.007	1	-5.51e-4	1	8695.476	4	NC	1
451	17	max	.006	3	-.002	15	.212	4	1.203e-2	4	NC	1	NC	1
452		min	-.005	2	-.01	4	-.009	1	-5.874e-4	1	NC	1	NC	1
453	18	max	.007	3	-.002	15	.224	4	1.283e-2	4	NC	1	NC	2
454		min	-.005	2	-.006	4	-.011	1	-6.237e-4	1	NC	1	9281.402	1
455	19	max	.007	3	0	10	.238	4	1.363e-2	4	NC	1	NC	2
456		min	-.006	2	-.002	1	-.013	1	-6.601e-4	1	NC	1	7743.392	1
457	M12	1	max	.003	1	.005	.013	1	-1.321e-5	12	NC	1	NC	3
458		min	0	3	-.007	3	-.238	4	-7.454e-4	4	NC	1	104.017	4
459	2	max	.002	1	.005	2	.012	1	-1.321e-5	12	NC	1	NC	3
460		min	0	3	-.007	3	-.219	4	-7.454e-4	4	NC	1	113.151	4
461	3	max	.002	1	.005	2	.011	1	-1.321e-5	12	NC	1	NC	3
462		min	0	3	-.007	3	-.2	4	-7.454e-4	4	NC	1	124.019	4
463	4	max	.002	1	.004	2	.01	1	-1.321e-5	12	NC	1	NC	3
464		min	0	3	-.006	3	-.181	4	-7.454e-4	4	NC	1	137.072	4
465	5	max	.002	1	.004	2	.009	1	-1.321e-5	12	NC	1	NC	3
466		min	0	3	-.006	3	-.162	4	-7.454e-4	4	NC	1	152.923	4
467	6	max	.002	1	.004	2	.008	1	-1.321e-5	12	NC	1	NC	3
468		min	0	3	-.005	3	-.144	4	-7.454e-4	4	NC	1	172.42	4
469	7	max	.002	1	.004	2	.007	1	-1.321e-5	12	NC	1	NC	3
470		min	0	3	-.005	3	-.126	4	-7.454e-4	4	NC	1	196.77	4
471	8	max	.002	1	.003	2	.006	1	-1.321e-5	12	NC	1	NC	3
472		min	0	3	-.005	3	-.109	4	-7.454e-4	4	NC	1	227.732	4
473	9	max	.001	1	.003	2	.005	1	-1.321e-5	12	NC	1	NC	2
474		min	0	3	-.004	3	-.093	4	-7.454e-4	4	NC	1	267.955	4
475	10	max	.001	1	.003	2	.004	1	-1.321e-5	12	NC	1	NC	2
476		min	0	3	-.004	3	-.077	4	-7.454e-4	4	NC	1	321.577	4
477	11	max	.001	1	.002	2	.004	1	-1.321e-5	12	NC	1	NC	2
478		min	0	3	-.003	3	-.063	4	-7.454e-4	4	NC	1	395.37	4
479	12	max	.001	1	.002	2	.003	1	-1.321e-5	12	NC	1	NC	2
480		min	0	3	-.003	3	-.05	4	-7.454e-4	4	NC	1	501.049	4
481	13	max	0	1	.002	2	.002	1	-1.321e-5	12	NC	1	NC	1
482		min	0	3	-.002	3	-.038	4	-7.454e-4	4	NC	1	660.358	4
483	14	max	0	1	.001	2	.002	1	-1.321e-5	12	NC	1	NC	1
484		min	0	3	-.002	3	-.027	4	-7.454e-4	4	NC	1	917.533	4
485	15	max	0	1	.001	2	.001	1	-1.321e-5	12	NC	1	NC	1
486		min	0	3	-.002	3	-.018	4	-7.454e-4	4	NC	1	1374.545	4
487	16	max	0	1	0	2	0	1	-1.321e-5	12	NC	1	NC	1
488		min	0	3	-.001	3	-.011	4	-7.454e-4	4	NC	1	2313.286	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-1.321e-5	12	NC	1	NC	1
490			min	0	3	0	3	-.005	4	-7.454e-4	4	NC	1	4778.943	4
491		18	max	0	1	0	2	0	1	-1.321e-5	12	NC	1	NC	1
492			min	0	3	0	3	-.002	4	-7.454e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-1.321e-5	12	NC	1	NC	1
494			min	0	1	0	1	0	1	-7.454e-4	4	NC	1	NC	1
495	M1	1	max	.009	3	.168	2	1.035	4	1.334e-2	1	NC	1	NC	1
496			min	-.005	2	-.032	3	0	12	-2.168e-2	3	NC	1	NC	1
497		2	max	.009	3	.081	2	1	4	1.033e-2	4	NC	5	NC	1
498			min	-.005	2	-.014	3	-.01	1	-1.076e-2	3	1568.567	2	8154.439	5
499		3	max	.009	3	.014	3	.964	4	1.769e-2	4	NC	5	NC	2
500			min	-.005	2	-.012	2	-.014	1	-2.995e-4	1	756.929	2	4499.566	5
501		4	max	.009	3	.06	3	.926	4	1.536e-2	4	NC	15	NC	2
502			min	-.005	2	-.115	2	-.013	1	-4.388e-3	3	479.151	2	3267.96	5
503		5	max	.009	3	.118	3	.887	4	1.303e-2	4	9888.605	15	NC	1
504			min	-.004	2	-.223	2	-.009	1	-8.665e-3	3	346.439	2	2649.039	5
505		6	max	.009	3	.181	3	.847	4	1.401e-2	1	7801.805	15	NC	1
506			min	-.004	2	-.328	2	-.004	1	-1.294e-2	3	273.204	2	2272.759	5
507		7	max	.008	3	.242	3	.807	4	1.878e-2	1	6571.235	15	NC	1
508			min	-.004	2	-.421	2	0	3	-1.722e-2	3	229.93	2	1998.653	4
509		8	max	.008	3	.292	3	.765	4	2.355e-2	1	5843.791	15	NC	1
510			min	-.004	2	-.496	2	0	12	-2.15e-2	3	204.322	2	1791.765	4
511		9	max	.008	3	.325	3	.722	4	2.603e-2	1	5463.678	15	NC	1
512			min	-.004	2	-.542	2	0	1	-2.178e-2	3	190.973	2	1654.548	4
513		10	max	.008	3	.337	3	.675	4	2.725e-2	2	5347.623	15	NC	1
514			min	-.004	2	-.558	2	0	12	-1.941e-2	3	187.053	2	1613.544	4
515		11	max	.008	3	.329	3	.624	4	2.913e-2	2	5463.429	15	NC	1
516			min	-.004	2	-.542	2	0	12	-1.703e-2	3	191.618	2	1648.407	4
517		12	max	.007	3	.301	3	.569	4	2.804e-2	2	5843.207	15	NC	1
518			min	-.004	2	-.493	2	-.001	1	-1.444e-2	3	206.192	1	1766.253	4
519		13	max	.007	3	.257	3	.508	4	2.252e-2	2	6570.101	15	NC	1
520			min	-.004	2	-.416	2	0	1	-1.155e-2	3	233.787	1	2097.599	4
521		14	max	.007	3	.2	3	.442	4	1.699e-2	2	7799.724	15	NC	1
522			min	-.004	2	-.319	2	0	12	-8.659e-3	3	280.838	1	2833.175	4
523		15	max	.007	3	.135	3	.374	4	1.146e-2	2	9884.782	15	NC	1
524			min	-.004	2	-.212	2	0	12	-5.769e-3	3	361.45	1	4587.604	4
525		16	max	.007	3	.069	3	.309	4	1.079e-2	4	NC	15	NC	1
526			min	-.004	2	-.105	2	0	12	-2.878e-3	3	509.72	1	NC	1
527		17	max	.006	3	.005	3	.248	4	1.208e-2	4	NC	5	NC	2
528			min	-.004	2	-.006	2	0	12	1.174e-5	12	824.041	1	9535.805	1
529		18	max	.006	3	.081	1	.194	4	9.339e-3	2	NC	5	NC	1
530			min	-.004	2	-.053	3	0	12	-3.353e-3	3	1736.687	1	NC	1
531		19	max	.006	3	.158	1	.148	4	1.853e-2	2	NC	1	NC	1
532			min	-.004	2	-.106	3	-.001	1	-6.831e-3	3	NC	1	NC	1
533	M5	1	max	.028	3	.352	2	1.035	4	0	1	NC	1	NC	1
534			min	-.019	2	-.029	3	0	1	-9.747e-6	4	NC	1	NC	1
535		2	max	.028	3	.169	2	1.008	4	9.078e-3	4	NC	5	NC	1
536			min	-.02	2	-.009	3	0	1	0	1	746.624	2	6030.126	4
537		3	max	.028	3	.043	3	.974	4	1.795e-2	4	NC	15	NC	1
538			min	-.02	2	-.037	2	0	1	0	1	350.606	2	3566.54	4
539		4	max	.028	3	.157	3	.935	4	1.462e-2	4	6861.793	15	NC	1
540			min	-.019	2	-.283	2	0	1	0	1	214.333	2	2787.746	4
541		5	max	.027	3	.313	3	.893	4	1.13e-2	4	4786.947	15	NC	1
542			min	-.019	2	-.551	2	0	1	0	1	150.594	2	2422.273	4
543		6	max	.026	3	.488	3	.85	4	7.976e-3	4	3676.477	15	NC	1
544			min	-.018	2	-.817	2	0	1	0	1	116.231	2	2200.009	4
545		7	max	.026	3	.659	3	.806	4	4.652e-3	4	3036.624	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.018	2	-1.059	2	0	1	0	1	96.317	2	2016.93	4
547		8	max	.025	3	.803	3	.764	4	1.328e-3	4	2665.345	15	NC	1
548			min	-.018	2	-1.252	2	0	1	0	1	84.71	1	1822.187	4
549		9	max	.025	3	.895	3	.722	4	0	1	2475.004	15	NC	1
550			min	-.017	2	-1.375	2	0	1	-5.859e-6	5	78.64	1	1648.279	4
551		10	max	.024	3	.929	3	.674	4	0	1	2417.616	15	NC	1
552			min	-.017	2	-1.417	2	0	1	-5.651e-6	5	76.838	1	1626.409	4
553		11	max	.023	3	.906	3	.623	4	0	1	2475.104	15	NC	1
554			min	-.017	2	-1.375	2	0	1	-5.443e-6	5	78.786	1	1672.588	4
555		12	max	.023	3	.827	3	.571	4	8.441e-4	4	2665.589	15	NC	1
556			min	-.016	2	-1.247	2	0	1	0	1	85.192	1	1730.609	4
557		13	max	.022	3	.7	3	.51	4	2.958e-3	4	3037.136	15	NC	1
558			min	-.016	2	-1.043	2	0	1	0	1	97.807	1	2048.764	4
559		14	max	.022	3	.541	3	.441	4	5.072e-3	4	3677.495	15	NC	1
560			min	-.016	2	-.791	1	0	1	0	1	119.788	1	2935.086	4
561		15	max	.021	3	.363	3	.369	4	7.186e-3	4	4788.986	15	NC	1
562			min	-.016	2	-.519	1	0	1	0	1	158.521	1	5797.712	4
563		16	max	.02	3	.182	3	.299	4	9.3e-3	4	6866.103	15	NC	1
564			min	-.015	2	-.251	1	0	1	0	1	232.359	1	NC	1
565		17	max	.02	3	.014	3	.237	4	1.141e-2	4	NC	15	NC	1
566			min	-.015	2	-.019	2	0	1	0	1	394.731	1	NC	1
567		18	max	.02	3	.169	1	.186	4	5.773e-3	4	NC	5	NC	1
568			min	-.015	2	-.129	3	0	1	0	1	865.733	1	NC	1
569		19	max	.02	3	.323	1	.149	4	0	1	NC	1	NC	1
570			min	-.015	2	-.258	3	0	1	-5.737e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.168	2	1.035	4	2.168e-2	3	NC	1	NC	1
572			min	-.005	2	-.032	3	-.001	1	-1.334e-2	1	NC	1	NC	1
573		2	max	.009	3	.081	2	1.006	4	1.076e-2	3	NC	5	NC	1
574			min	-.005	2	-.014	3	0	12	-6.418e-3	1	1568.567	2	6427.583	4
575		3	max	.009	3	.014	3	.972	4	1.789e-2	4	NC	5	NC	2
576			min	-.005	2	-.012	2	0	12	4.601e-6	10	756.929	2	3737.116	4
577		4	max	.009	3	.06	3	.934	4	1.4e-2	5	NC	15	NC	2
578			min	-.005	2	-.115	2	0	12	-4.471e-3	1	479.151	2	2866.034	4
579		5	max	.009	3	.118	3	.893	4	1.055e-2	5	9840.069	15	NC	1
580			min	-.004	2	-.223	2	0	12	-9.241e-3	1	346.439	2	2445.319	4
581		6	max	.009	3	.181	3	.85	4	1.294e-2	3	7765.156	15	NC	1
582			min	-.004	2	-.328	2	0	12	-1.401e-2	1	273.204	2	2189.304	4
583		7	max	.008	3	.242	3	.807	4	1.722e-2	3	6541.38	15	NC	1
584			min	-.004	2	-.421	2	0	1	-1.878e-2	1	229.93	2	1993.452	4
585		8	max	.008	3	.292	3	.764	4	2.15e-2	3	5817.856	15	NC	1
586			min	-.004	2	-.496	2	-.001	1	-2.355e-2	1	204.322	2	1808.153	4
587		9	max	.008	3	.325	3	.722	4	2.178e-2	3	5439.744	15	NC	1
588			min	-.004	2	-.542	2	0	12	-2.603e-2	1	190.973	2	1648.572	4
589		10	max	.008	3	.337	3	.675	4	1.941e-2	3	5324.28	15	NC	1
590			min	-.004	2	-.558	2	0	1	-2.725e-2	2	187.053	2	1614.715	4
591		11	max	.008	3	.329	3	.623	4	1.703e-2	3	5439.515	15	NC	1
592			min	-.004	2	-.542	2	0	1	-2.913e-2	2	191.618	2	1656.189	4
593		12	max	.007	3	.301	3	.57	4	1.444e-2	3	5817.444	15	NC	1
594			min	-.004	2	-.493	2	0	12	-2.804e-2	2	206.192	1	1750.403	4
595		13	max	.007	3	.257	3	.508	4	1.155e-2	3	6540.795	15	NC	1
596			min	-.004	2	-.416	2	0	12	-2.252e-2	2	233.787	1	2097.986	4
597		14	max	.007	3	.2	3	.44	4	8.659e-3	3	7764.356	15	NC	1
598			min	-.004	2	-.319	2	-.003	1	-1.699e-2	2	280.838	1	2942.447	5
599		15	max	.007	3	.135	3	.369	4	6.832e-3	5	9838.952	15	NC	1
600			min	-.004	2	-.212	2	-.008	1	-1.146e-2	2	361.45	1	5191.418	5
601		16	max	.007	3	.069	3	.301	4	9.198e-3	5	NC	15	NC	1
602			min	-.004	2	-.105	2	-.012	1	-5.959e-3	1	509.72	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.006	3	.005	3	.239	4	1.159e-2	4	NC	5	NC	2
604		min	-.004	2	-.006	2	-.013	1	-8.473e-4	1	824.041	1	9535.805	1
605	18	max	.006	3	.081	1	.189	4	5.527e-3	5	NC	5	NC	1
606		min	-.004	2	-.053	3	-.009	1	-9.339e-3	2	1736.687	1	NC	1
607	19	max	.006	3	.158	1	.149	4	6.831e-3	3	NC	1	NC	1
608		min	-.004	2	-.106	3	0	12	-1.853e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

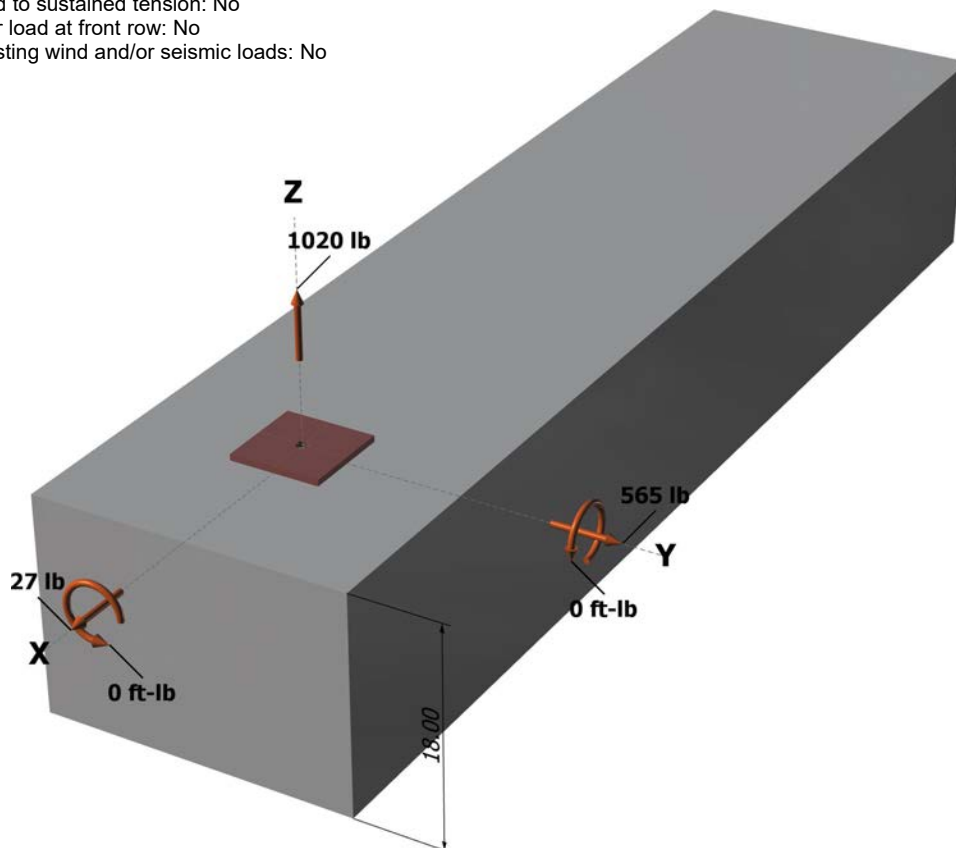
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
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Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



# Anchor Designer™ Software Version 2.4.6025.0

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Address:			
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

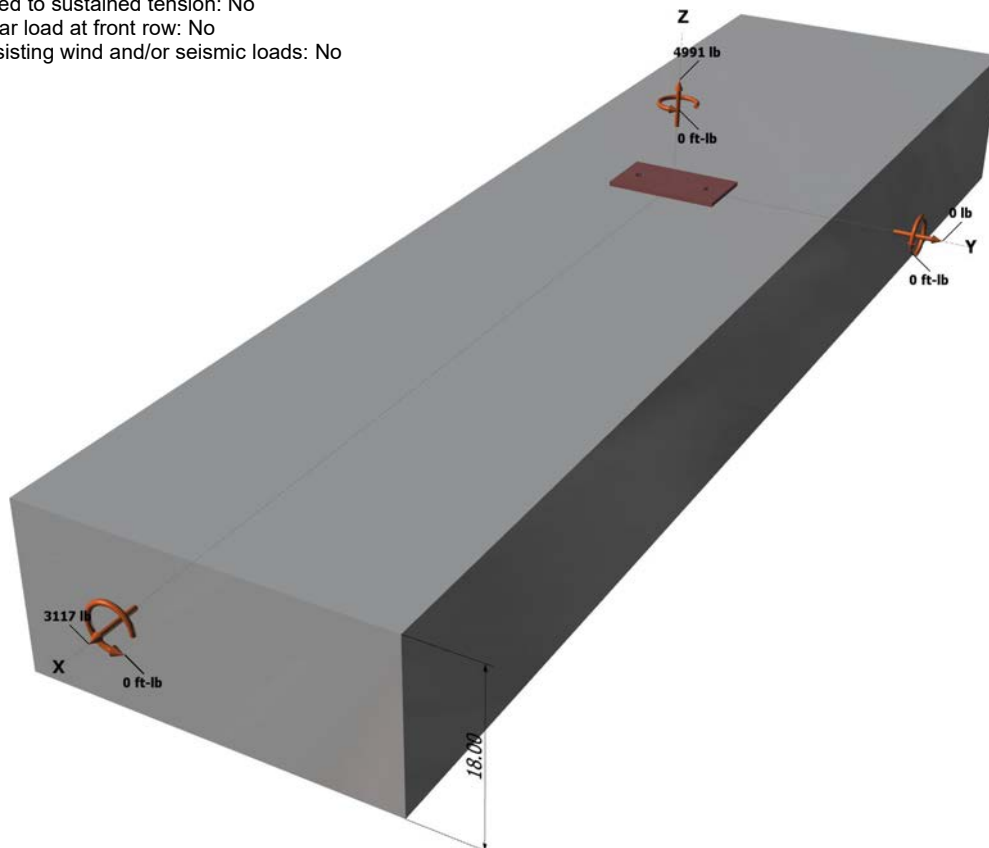
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

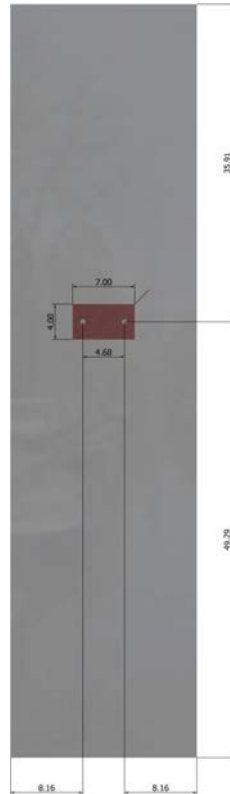
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

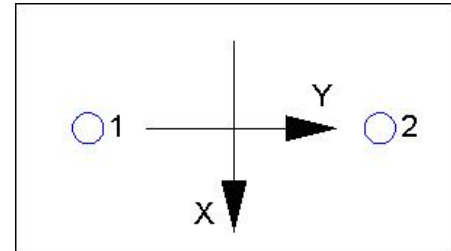
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.