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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

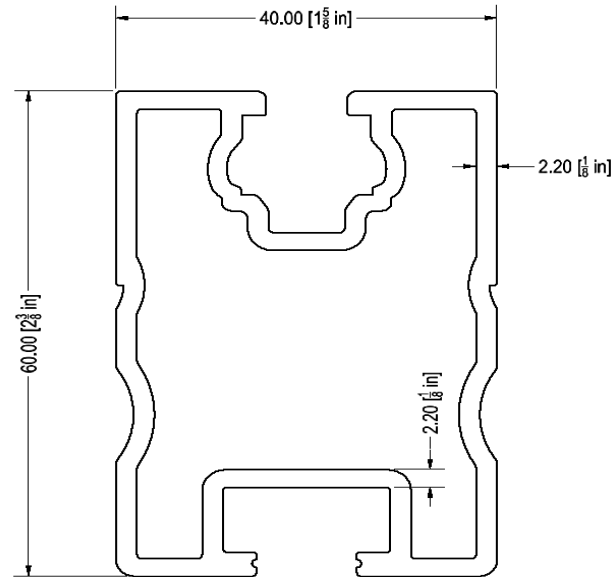
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	63 in
$\Phi F_{ty}$ STRONG-AXIS =	29.20 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.468 k-ft
$M_z$ =	0.113 k-ft
$M_{y \text{ allowable}}$ =	1.243 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>51%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.41 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.557 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.287 k
$M_{y \text{ allowable}}$ =	1.443 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>41%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.894 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.603 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.741 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>15%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.077 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>7%</b>



A cross brace kit is required every 27 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

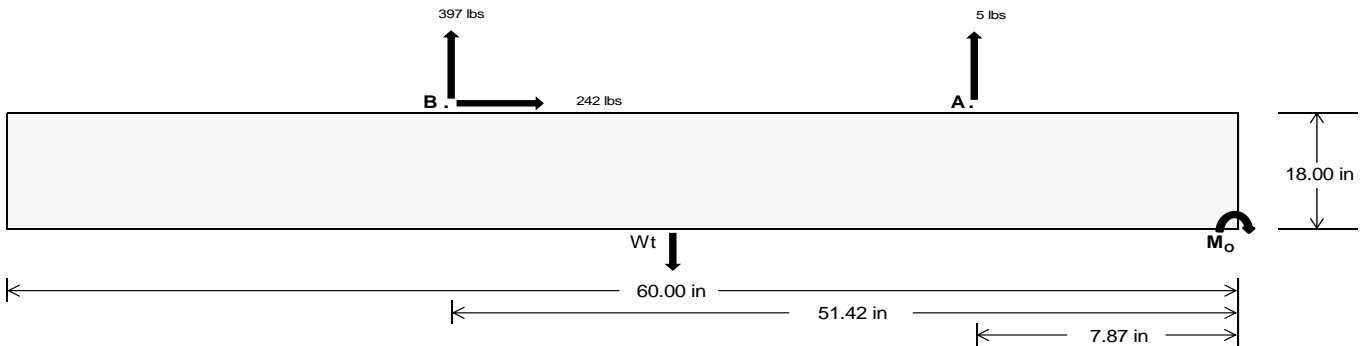
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>28.47</u>	<u>1723.35</u>	k
Compressive Load =	<u>1162.78</u>	<u>1203.19</u>	k
Lateral Load =	<u>2.26</u>	<u>1047.71</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 24789.7$  in-lbs  
Resisting Force Required = 826.32 lbs  
S.F. = 1.67  
Weight Required = 1377.21 lbs  
Minimum Width = 20 in  
Weight Provided = 1812.50 lbs

### Sliding

Force = 241.65 lbs  
Friction = 0.4  
Weight Required = 604.12 lbs  
Resisting Weight = 1812.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 241.65 lbs  
Cohesion = 130 psf  
Area = 8.33 ft<sup>2</sup>  
Resisting = 906.25 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width	20 in	21 in	22 in	23 in
	1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
$F_A$	418 lbs	418 lbs	418 lbs	418 lbs	383 lbs	383 lbs	383 lbs	383 lbs	563 lbs	563 lbs	563 lbs	563 lbs	-10 lbs	-10 lbs	-10 lbs	-10 lbs
$F_B$	289 lbs	289 lbs	289 lbs	289 lbs	500 lbs	500 lbs	500 lbs	500 lbs	564 lbs	564 lbs	564 lbs	564 lbs	-794 lbs	-794 lbs	-794 lbs	-794 lbs
$F_V$	45 lbs	45 lbs	45 lbs	45 lbs	437 lbs	437 lbs	437 lbs	437 lbs	358 lbs	358 lbs	358 lbs	358 lbs	-483 lbs	-483 lbs	-483 lbs	-483 lbs
$P_{total}$	2520 lbs	2610 lbs	2701 lbs	2792 lbs	2696 lbs	2786 lbs	2877 lbs	2968 lbs	2940 lbs	3031 lbs	3121 lbs	3212 lbs	284 lbs	338 lbs	393 lbs	447 lbs
$M$	324 lbs-ft	324 lbs-ft	324 lbs-ft	324 lbs-ft	469 lbs-ft	469 lbs-ft	469 lbs-ft	469 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	673 lbs-ft	673 lbs-ft	673 lbs-ft	673 lbs-ft
$e$	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.37 ft	1.99 ft	1.71 ft	1.50 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	255.7 psf	253.9 psf	252.3 psf	250.7 psf	255.9 psf	254.1 psf	252.4 psf	250.9 psf	271.1 psf	268.5 psf	266.2 psf	264.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	349.0 psf	342.7 psf	337.0 psf	331.8 psf	391.1 psf	382.8 psf	375.3 psf	368.4 psf	434.5 psf	424.2 psf	414.8 psf	406.2 psf	870.5 psf	252.0 psf	181.4 psf	156.2 psf

Maximum Bearing Pressure = 870 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

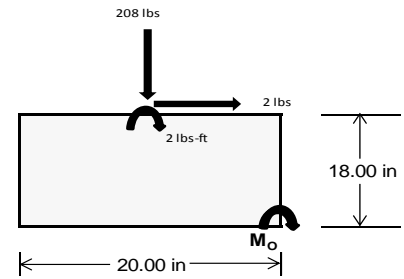
## Overturning Check

$M_o = 167.5 \text{ ft-lbs}$   
 Resisting Force Required = 200.97 lbs  
 S.F. = 1.67  
 Weight Required = 334.96 lbs  
 Minimum Width = 20 in  
 Weight Provided = 1812.50 lbs

*A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	65 lbs	162 lbs	61 lbs	208 lbs	586 lbs	204 lbs	19 lbs	47 lbs	18 lbs
$F_v$	0 lbs	0 lbs	0 lbs	2 lbs	2 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2309 lbs	2406 lbs	2305 lbs	2344 lbs	2722 lbs	2340 lbs	675 lbs	703 lbs	674 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	6 lbs-ft	3 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.66 ft	1.66 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
$f_{min}$	276.8 sqft	288.5 sqft	276.5 sqft	278.8 sqft	325.4 sqft	280.5 sqft	80.9 sqft	84.4 sqft	80.9 sqft
$f_{max}$	277.2 psf	288.8 psf	276.7 psf	283.7 psf	328.0 psf	281.1 psf	81.1 psf	84.4 psf	80.9 psf



Maximum Bearing Pressure = 328 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



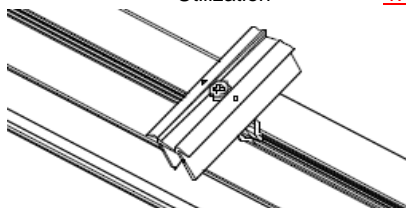
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

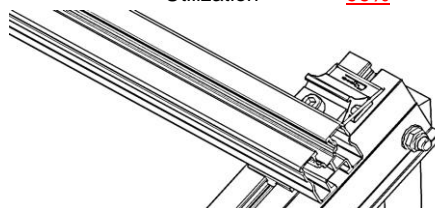
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.571 k
Allowable Uplift =	1.214 k
Utilization =	<u>47%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.067 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



### 6.2 Bolted Connections

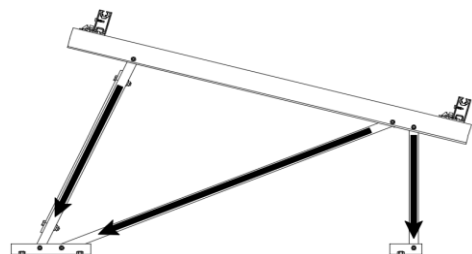
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.894 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

#### Diagonal Strut

Maximum Axial Load =	0.603 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.114 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.077 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.016 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$164.048$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$170.354$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.2 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.243 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.10 \\ &23.4092 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.4 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.10 \\ &24.5845 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.4 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.443 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	211.714	2	279.876	2	0	15	0	15	0	1	0	1
2		min	-257.294	3	-408.359	3	-.121	3	0	3	0	1	0	1
3	N7	max	.002	3	333.204	1	-.035	15	0	15	0	1	0	1
4		min	-.139	2	4.114	12	-.812	1	-.001	1	0	1	0	1
5	N15	max	0	15	894.444	1	.409	1	0	1	0	1	0	1
6		min	-1.414	2	-21.897	3	-.544	3	0	3	0	1	0	1
7	N16	max	743.646	2	925.531	2	0	10	0	1	0	1	0	1
8		min	-805.932	3	-1325.653	3	-64.496	3	0	3	0	1	0	1
9	N23	max	.002	3	333.054	1	1.74	1	.003	1	0	1	0	1
10		min	-.139	2	4.472	12	.078	15	0	15	0	1	0	1
11	N24	max	211.79	2	282.823	2	65.004	3	0	1	0	1	0	1
12		min	-257.598	3	-406.781	3	.005	10	0	3	0	1	0	1
13	Totals:	max	1165.458	2	2833.583	1	0	1						
14		min	-1320.853	3	-2152.771	3	0	15						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	231.703	1	.655	4	.271	1	0	15	0	15	0	1
2			min	-351.391	3	.154	15	-.056	3	0	1	0	1	0	1
3		2	max	231.829	1	.604	4	.271	1	0	15	0	15	0	15
4			min	-351.297	3	.142	15	-.056	3	0	1	0	1	0	4
5		3	max	231.955	1	.553	4	.271	1	0	15	0	15	0	15
6			min	-351.202	3	.13	15	-.056	3	0	1	0	3	0	4
7		4	max	232.08	1	.501	4	.271	1	0	15	0	1	0	15
8			min	-351.108	3	.118	15	-.056	3	0	1	0	3	0	4
9		5	max	232.206	1	.45	4	.271	1	0	15	0	1	0	15
10			min	-351.014	3	.106	15	-.056	3	0	1	0	3	0	4
11		6	max	232.332	1	.399	4	.271	1	0	15	0	1	0	15
12			min	-350.919	3	.094	15	-.056	3	0	1	0	3	0	4
13		7	max	232.458	1	.348	4	.271	1	0	15	0	1	0	15
14			min	-350.825	3	.082	15	-.056	3	0	1	0	3	0	4
15		8	max	232.584	1	.297	4	.271	1	0	15	0	1	0	15
16			min	-350.73	3	.07	15	-.056	3	0	1	0	3	0	4
17		9	max	232.71	1	.246	4	.271	1	0	15	0	1	0	15
18			min	-350.636	3	.058	15	-.056	3	0	1	0	3	0	4
19		10	max	232.836	1	.195	4	.271	1	0	15	0	1	0	15
20			min	-350.542	3	.046	15	-.056	3	0	1	0	3	0	4
21		11	max	232.961	1	.144	4	.271	1	0	15	0	1	0	15
22			min	-350.447	3	.034	15	-.056	3	0	1	0	3	0	4
23		12	max	233.087	1	.101	2	.271	1	0	15	0	1	0	15
24			min	-350.353	3	.014	12	-.056	3	0	1	0	3	0	4
25		13	max	233.213	1	.061	2	.271	1	0	15	0	1	0	15
26			min	-350.258	3	-.012	3	-.056	3	0	1	0	3	0	4
27		14	max	233.339	1	.021	2	.271	1	0	15	0	1	0	15
28			min	-350.164	3	-.042	3	-.056	3	0	1	0	3	0	4
29		15	max	233.465	1	-.014	15	.271	1	0	15	0	1	0	15
30			min	-350.07	3	-.072	3	-.056	3	0	1	0	3	0	4
31		16	max	233.591	1	-.026	15	.271	1	0	15	0	1	0	15
32			min	-349.975	3	-.112	4	-.056	3	0	1	0	3	0	4
33		17	max	233.717	1	-.038	15	.271	1	0	15	0	1	0	15
34			min	-349.881	3	-.163	4	-.056	3	0	1	0	3	0	4
35		18	max	233.843	1	-.05	15	.271	1	0	15	0	1	0	15
36			min	-349.786	3	-.214	4	-.056	3	0	1	0	3	0	4
37		19	max	233.968	1	-.062	15	.271	1	0	15	0	1	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-349.692	3	-.266	4	-.056	3	0	1	0	3	0	4
39	M3	1	max	165.618	2	1.758	4	-.012	15	0	15	0	1	0	4
40			min	-169.057	3	.413	15	-.286	1	0	1	0	15	0	15
41		2	max	165.549	2	1.581	4	-.012	15	0	15	0	1	0	2
42			min	-169.109	3	.372	15	-.286	1	0	1	0	15	0	12
43		3	max	165.479	2	1.404	4	-.012	15	0	15	0	1	0	2
44			min	-169.161	3	.33	15	-.286	1	0	1	0	15	0	3
45		4	max	165.41	2	1.228	4	-.012	15	0	15	0	1	0	15
46			min	-169.213	3	.289	15	-.286	1	0	1	0	15	0	4
47		5	max	165.341	2	1.051	4	-.012	15	0	15	0	1	0	15
48			min	-169.265	3	.247	15	-.286	1	0	1	0	15	0	4
49		6	max	165.271	2	.874	4	-.012	15	0	15	0	1	0	15
50			min	-169.317	3	.206	15	-.286	1	0	1	0	15	0	4
51		7	max	165.202	2	.697	4	-.012	15	0	15	0	1	0	15
52			min	-169.369	3	.164	15	-.286	1	0	1	0	15	0	4
53		8	max	165.133	2	.52	4	-.012	15	0	15	0	1	0	15
54			min	-169.421	3	.122	15	-.286	1	0	1	0	15	-.001	4
55		9	max	165.063	2	.343	4	-.012	15	0	15	0	1	0	15
56			min	-169.473	3	.081	15	-.286	1	0	1	0	15	-.001	4
57		10	max	164.994	2	.167	4	-.012	15	0	15	0	1	0	15
58			min	-169.525	3	.039	15	-.286	1	0	1	0	15	-.001	4
59		11	max	164.925	2	.017	2	-.012	15	0	15	0	1	0	15
60			min	-169.577	3	-.037	3	-.286	1	0	1	0	15	-.001	4
61		12	max	164.855	2	-.044	15	-.012	15	0	15	0	1	0	15
62			min	-169.629	3	-.187	4	-.286	1	0	1	0	15	-.001	4
63		13	max	164.786	2	-.085	15	-.012	15	0	15	0	1	0	15
64			min	-169.681	3	-.364	4	-.286	1	0	1	0	15	-.001	4
65		14	max	164.717	2	-.127	15	-.012	15	0	15	0	1	0	15
66			min	-169.733	3	-.541	4	-.286	1	0	1	0	15	-.001	4
67		15	max	164.647	2	-.168	15	-.012	15	0	15	0	1	0	15
68			min	-169.785	3	-.718	4	-.286	1	0	1	0	10	0	4
69		16	max	164.578	2	-.21	15	-.012	15	0	15	0	1	0	15
70			min	-169.837	3	-.894	4	-.286	1	0	1	0	2	0	4
71		17	max	164.509	2	-.252	15	-.012	15	0	15	0	15	0	15
72			min	-169.889	3	-1.071	4	-.286	1	0	1	0	1	0	4
73		18	max	164.439	2	-.293	15	-.012	15	0	15	0	15	0	15
74			min	-169.941	3	-1.248	4	-.286	1	0	1	0	1	0	4
75		19	max	164.37	2	-.335	15	-.012	15	0	15	0	15	0	1
76			min	-169.993	3	-1.425	4	-.286	1	0	1	0	1	0	1
77	M4	1	max	332.039	1	0	1	-.036	15	0	1	0	3	0	1
78			min	3.531	12	0	1	-.86	1	0	1	0	2	0	1
79		2	max	332.104	1	0	1	-.036	15	0	1	0	15	0	1
80			min	3.564	12	0	1	-.86	1	0	1	0	1	0	1
81		3	max	332.168	1	0	1	-.036	15	0	1	0	15	0	1
82			min	3.596	12	0	1	-.86	1	0	1	0	1	0	1
83		4	max	332.233	1	0	1	-.036	15	0	1	0	15	0	1
84			min	3.628	12	0	1	-.86	1	0	1	0	1	0	1
85		5	max	332.298	1	0	1	-.036	15	0	1	0	15	0	1
86			min	3.661	12	0	1	-.86	1	0	1	0	1	0	1
87		6	max	332.362	1	0	1	-.036	15	0	1	0	15	0	1
88			min	3.693	12	0	1	-.86	1	0	1	0	1	0	1
89		7	max	332.427	1	0	1	-.036	15	0	1	0	15	0	1
90			min	3.725	12	0	1	-.86	1	0	1	0	1	0	1
91		8	max	332.492	1	0	1	-.036	15	0	1	0	15	0	1
92			min	3.758	12	0	1	-.86	1	0	1	0	1	0	1
93		9	max	332.556	1	0	1	-.036	15	0	1	0	15	0	1
94			min	3.79	12	0	1	-.86	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	332.621	1	0	1	-.036	15	0	1	0	15	0	1
96			min	3.822	12	0	1	-.86	1	0	1	0	1	0	1
97		11	max	332.686	1	0	1	-.036	15	0	1	0	15	0	1
98			min	3.855	12	0	1	-.86	1	0	1	0	1	0	1
99		12	max	332.751	1	0	1	-.036	15	0	1	0	15	0	1
100			min	3.887	12	0	1	-.86	1	0	1	0	1	0	1
101		13	max	332.815	1	0	1	-.036	15	0	1	0	15	0	1
102			min	3.919	12	0	1	-.86	1	0	1	0	1	0	1
103		14	max	332.88	1	0	1	-.036	15	0	1	0	15	0	1
104			min	3.952	12	0	1	-.86	1	0	1	-.001	1	0	1
105		15	max	332.945	1	0	1	-.036	15	0	1	0	15	0	1
106			min	3.984	12	0	1	-.86	1	0	1	-.001	1	0	1
107		16	max	333.009	1	0	1	-.036	15	0	1	0	15	0	1
108			min	4.017	12	0	1	-.86	1	0	1	-.001	1	0	1
109		17	max	333.074	1	0	1	-.036	15	0	1	0	15	0	1
110			min	4.049	12	0	1	-.86	1	0	1	-.001	1	0	1
111		18	max	333.139	1	0	1	-.036	15	0	1	0	15	0	1
112			min	4.081	12	0	1	-.86	1	0	1	-.001	1	0	1
113		19	max	333.204	1	0	1	-.036	15	0	1	0	15	0	1
114			min	4.114	12	0	1	-.86	1	0	1	-.001	1	0	1
115	M6	1	max	738.701	1	.656	4	.075	9	0	3	0	3	0	1
116			min	-1113.918	3	.154	15	-.202	3	0	10	0	9	0	1
117		2	max	738.827	1	.605	4	.075	9	0	3	0	3	0	15
118			min	-1113.823	3	.142	15	-.202	3	0	10	0	9	0	4
119		3	max	738.953	1	.554	4	.075	9	0	3	0	3	0	15
120			min	-1113.729	3	.13	15	-.202	3	0	10	0	9	0	4
121		4	max	739.079	1	.503	4	.075	9	0	3	0	3	0	15
122			min	-1113.635	3	.118	15	-.202	3	0	10	0	10	0	4
123		5	max	739.205	1	.452	4	.075	9	0	3	0	1	0	15
124			min	-1113.54	3	.106	15	-.202	3	0	10	0	10	0	4
125		6	max	739.331	1	.406	2	.075	9	0	3	0	1	0	15
126			min	-1113.446	3	.088	12	-.202	3	0	10	0	10	0	4
127		7	max	739.456	1	.366	2	.075	9	0	3	0	1	0	15
128			min	-1113.351	3	.068	12	-.202	3	0	10	0	3	0	4
129		8	max	739.582	1	.326	2	.075	9	0	3	0	1	0	15
130			min	-1113.257	3	.048	12	-.202	3	0	10	0	3	0	4
131		9	max	739.708	1	.286	2	.075	9	0	3	0	1	0	15
132			min	-1113.163	3	.028	12	-.202	3	0	10	0	3	0	4
133		10	max	739.834	1	.246	2	.075	9	0	3	0	1	0	12
134			min	-1113.068	3	.001	3	-.202	3	0	10	0	3	0	4
135		11	max	739.96	1	.207	2	.075	9	0	3	0	1	0	12
136			min	-1112.974	3	-.029	3	-.202	3	0	10	0	3	0	2
137		12	max	740.086	1	.167	2	.075	9	0	3	0	1	0	12
138			min	-1112.879	3	-.059	3	-.202	3	0	10	0	3	0	2
139		13	max	740.212	1	.127	2	.075	9	0	3	0	1	0	12
140			min	-1112.785	3	-.089	3	-.202	3	0	10	0	3	0	2
141		14	max	740.338	1	.087	2	.075	9	0	3	0	1	0	12
142			min	-1112.691	3	-.118	3	-.202	3	0	10	0	3	0	2
143		15	max	740.463	1	.047	2	.075	9	0	3	0	1	0	12
144			min	-1112.596	3	-.148	3	-.202	3	0	10	0	3	0	2
145		16	max	740.589	1	.007	2	.075	9	0	3	0	1	0	12
146			min	-1112.502	3	-.178	3	-.202	3	0	10	0	3	0	2
147		17	max	740.715	1	-.033	2	.075	9	0	3	0	1	0	12
148			min	-1112.407	3	-.208	3	-.202	3	0	10	0	3	0	2
149		18	max	740.841	1	-.05	15	.075	9	0	3	0	1	0	3
150			min	-1112.313	3	-.238	3	-.202	3	0	10	0	3	0	2
151		19	max	740.967	1	-.062	15	.075	9	0	3	0	1	0	3





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1112.219	3	-.268	3	-.202	3	0	10	0	3	0	2
153	M7	1	max	603.439	2	1.761	4	.026	3	0	1	0	2	2
154		min	-511.085	3	.414	15	-.015	2	0	3	0	3	0	3
155		2	max	603.369	2	1.584	4	.026	3	0	1	0	1	2
156		min	-511.137	3	.372	15	-.015	2	0	3	0	3	0	3
157		3	max	603.3	2	1.407	4	.026	3	0	1	0	1	2
158		min	-511.189	3	.331	15	-.015	2	0	3	0	3	0	3
159		4	max	603.231	2	1.231	4	.026	3	0	1	0	1	2
160		min	-511.241	3	.289	15	-.015	2	0	3	0	3	0	3
161		5	max	603.161	2	1.054	4	.026	3	0	1	0	1	15
162		min	-511.293	3	.248	15	-.015	2	0	3	0	3	0	3
163		6	max	603.092	2	.877	4	.026	3	0	1	0	1	15
164		min	-511.345	3	.206	15	-.015	2	0	3	0	3	0	4
165		7	max	603.023	2	.7	4	.026	3	0	1	0	1	15
166		min	-511.397	3	.165	15	-.015	2	0	3	0	3	0	4
167		8	max	602.953	2	.523	4	.026	3	0	1	0	1	15
168		min	-511.449	3	.123	15	-.015	2	0	3	0	3	-.001	4
169		9	max	602.884	2	.353	2	.026	3	0	1	0	1	15
170		min	-511.501	3	.075	12	-.015	2	0	3	0	3	-.001	4
171		10	max	602.815	2	.215	2	.026	3	0	1	0	1	15
172		min	-511.553	3	-.001	3	-.015	2	0	3	0	3	-.001	4
173		11	max	602.745	2	.078	2	.026	3	0	1	0	1	15
174		min	-511.605	3	-.105	3	-.015	2	0	3	0	3	-.001	4
175		12	max	602.676	2	-.043	15	.026	3	0	1	0	1	15
176		min	-511.657	3	-.208	3	-.015	2	0	3	0	3	-.001	4
177		13	max	602.607	2	-.085	15	.026	3	0	1	0	1	15
178		min	-511.709	3	-.361	4	-.015	2	0	3	0	3	-.001	4
179		14	max	602.537	2	-.126	15	.026	3	0	1	0	1	15
180		min	-511.761	3	-.538	4	-.015	2	0	3	0	3	-.001	4
181		15	max	602.468	2	-.168	15	.026	3	0	1	0	1	15
182		min	-511.813	3	-.715	4	-.015	2	0	3	0	3	0	4
183		16	max	602.399	2	-.21	15	.026	3	0	1	0	1	15
184		min	-511.865	3	-.891	4	-.015	2	0	3	0	3	0	4
185		17	max	602.33	2	-.251	15	.026	3	0	1	0	1	15
186		min	-511.917	3	-1.068	4	-.015	2	0	3	0	3	0	4
187		18	max	602.26	2	-.293	15	.026	3	0	1	0	1	15
188		min	-511.969	3	-1.245	4	-.015	2	0	3	0	3	0	4
189		19	max	602.191	2	-.334	15	.026	3	0	1	0	1	1
190		min	-512.021	3	-1.422	4	-.015	2	0	3	0	3	0	1
191	M8	1	max	893.279	1	0	1	.479	1	0	1	0	10	0
192		min	-22.77	3	0	1	-.542	3	0	1	0	1	0	1
193		2	max	893.344	1	0	1	.479	1	0	1	0	1	0
194		min	-22.722	3	0	1	-.542	3	0	1	0	3	0	1
195		3	max	893.409	1	0	1	.479	1	0	1	0	1	0
196		min	-22.673	3	0	1	-.542	3	0	1	0	3	0	1
197		4	max	893.474	1	0	1	.479	1	0	1	0	1	0
198		min	-22.625	3	0	1	-.542	3	0	1	0	3	0	1
199		5	max	893.538	1	0	1	.479	1	0	1	0	1	0
200		min	-22.576	3	0	1	-.542	3	0	1	0	3	0	1
201		6	max	893.603	1	0	1	.479	1	0	1	0	1	0
202		min	-22.527	3	0	1	-.542	3	0	1	0	3	0	1
203		7	max	893.668	1	0	1	.479	1	0	1	0	1	0
204		min	-22.479	3	0	1	-.542	3	0	1	0	3	0	1
205		8	max	893.732	1	0	1	.479	1	0	1	0	1	0
206		min	-22.43	3	0	1	-.542	3	0	1	0	3	0	1
207		9	max	893.797	1	0	1	.479	1	0	1	0	1	0
208		min	-22.382	3	0	1	-.542	3	0	1	0	3	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	893.862	1	0	1	.479	1	0	1	0	1	0	1
210			min	-22.333	3	0	1	-.542	3	0	1	0	3	0	1
211		11	max	893.926	1	0	1	.479	1	0	1	0	1	0	1
212			min	-22.285	3	0	1	-.542	3	0	1	0	3	0	1
213		12	max	893.991	1	0	1	.479	1	0	1	0	1	0	1
214			min	-22.236	3	0	1	-.542	3	0	1	0	3	0	1
215		13	max	894.056	1	0	1	.479	1	0	1	0	1	0	1
216			min	-22.188	3	0	1	-.542	3	0	1	0	3	0	1
217		14	max	894.121	1	0	1	.479	1	0	1	0	1	0	1
218			min	-22.139	3	0	1	-.542	3	0	1	0	3	0	1
219		15	max	894.185	1	0	1	.479	1	0	1	0	1	0	1
220			min	-22.091	3	0	1	-.542	3	0	1	0	3	0	1
221		16	max	894.25	1	0	1	.479	1	0	1	0	1	0	1
222			min	-22.042	3	0	1	-.542	3	0	1	0	3	0	1
223		17	max	894.315	1	0	1	.479	1	0	1	0	1	0	1
224			min	-21.994	3	0	1	-.542	3	0	1	0	3	0	1
225		18	max	894.379	1	0	1	.479	1	0	1	0	1	0	1
226			min	-21.945	3	0	1	-.542	3	0	1	0	3	0	1
227		19	max	894.444	1	0	1	.479	1	0	1	0	1	0	1
228			min	-21.897	3	0	1	-.542	3	0	1	0	3	0	1
229	M10	1	max	234.08	1	.651	4	-.004	15	0	1	0	1	0	1
230			min	-310.009	3	.153	15	-.131	1	0	3	0	3	0	1
231		2	max	234.205	1	.599	4	-.004	15	0	1	0	1	0	15
232			min	-309.914	3	.141	15	-.131	1	0	3	0	3	0	4
233		3	max	234.331	1	.548	4	-.004	15	0	1	0	1	0	15
234			min	-309.82	3	.129	15	-.131	1	0	3	0	3	0	4
235		4	max	234.457	1	.497	4	-.004	15	0	1	0	1	0	15
236			min	-309.725	3	.117	15	-.131	1	0	3	0	3	0	4
237		5	max	234.583	1	.446	4	-.004	15	0	1	0	1	0	15
238			min	-309.631	3	.105	15	-.131	1	0	3	0	3	0	4
239		6	max	234.709	1	.395	4	-.004	15	0	1	0	1	0	15
240			min	-309.537	3	.093	15	-.131	1	0	3	0	3	0	4
241		7	max	234.835	1	.344	4	-.004	15	0	1	0	1	0	15
242			min	-309.442	3	.081	15	-.131	1	0	3	0	3	0	4
243		8	max	234.961	1	.293	4	-.004	15	0	1	0	1	0	15
244			min	-309.348	3	.069	15	-.131	1	0	3	0	3	0	4
245		9	max	235.087	1	.241	4	-.004	15	0	1	0	9	0	15
246			min	-309.253	3	.057	15	-.131	1	0	3	0	3	0	4
247		10	max	235.212	1	.19	4	-.004	15	0	1	0	9	0	15
248			min	-309.159	3	.045	15	-.131	1	0	3	0	3	0	4
249		11	max	235.338	1	.14	2	-.004	15	0	1	0	9	0	15
250			min	-309.065	3	.033	15	-.131	1	0	3	0	3	0	4
251		12	max	235.464	1	.101	2	-.004	15	0	1	0	15	0	15
252			min	-308.97	3	.021	15	-.131	1	0	3	0	3	0	4
253		13	max	235.59	1	.061	2	-.004	15	0	1	0	15	0	15
254			min	-308.876	3	.008	12	-.131	1	0	3	0	3	0	4
255		14	max	235.716	1	.021	2	-.004	15	0	1	0	15	0	15
256			min	-308.781	3	-.019	3	-.131	1	0	3	0	3	0	4
257		15	max	235.842	1	-.015	15	-.004	15	0	1	0	15	0	15
258			min	-308.687	3	-.065	4	-.131	1	0	3	0	3	0	4
259		16	max	235.968	1	-.027	15	-.004	15	0	1	0	15	0	15
260			min	-308.593	3	-.117	4	-.131	1	0	3	0	3	0	4
261		17	max	236.093	1	-.039	15	-.004	15	0	1	0	15	0	15
262			min	-308.498	3	-.168	4	-.131	1	0	3	0	3	0	4
263		18	max	236.219	1	-.051	15	-.004	15	0	1	0	15	0	15
264			min	-308.404	3	-.219	4	-.131	1	0	3	0	1	0	4
265		19	max	236.345	1	-.063	15	-.004	15	0	1	0	15	0	15



Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-308.309	3	-.27	4	-.131	1	0	3	0	1	0	4
267	M11	1	max	165.183	2	1.761	4	.312	1	0	1	0	3	0	4
268			min	-169.723	3	.414	15	-.033	3	0	15	0	1	0	12
269		2	max	165.113	2	1.585	4	.312	1	0	1	0	3	0	2
270			min	-169.775	3	.372	15	-.033	3	0	15	0	1	0	3
271		3	max	165.044	2	1.408	4	.312	1	0	1	0	3	0	2
272			min	-169.827	3	.331	15	-.033	3	0	15	0	1	0	3
273		4	max	164.975	2	1.231	4	.312	1	0	1	0	3	0	15
274			min	-169.879	3	.289	15	-.033	3	0	15	0	1	0	3
275		5	max	164.905	2	1.054	4	.312	1	0	1	0	3	0	15
276			min	-169.931	3	.248	15	-.033	3	0	15	0	1	0	4
277		6	max	164.836	2	.877	4	.312	1	0	1	0	3	0	15
278			min	-169.983	3	.206	15	-.033	3	0	15	0	1	0	4
279		7	max	164.767	2	.7	4	.312	1	0	1	0	3	0	15
280			min	-170.035	3	.165	15	-.033	3	0	15	0	1	0	4
281		8	max	164.697	2	.524	4	.312	1	0	1	0	3	0	15
282			min	-170.087	3	.123	15	-.033	3	0	15	0	1	-.001	4
283		9	max	164.628	2	.347	4	.312	1	0	1	0	3	0	15
284			min	-170.139	3	.081	15	-.033	3	0	15	0	1	-.001	4
285		10	max	164.559	2	.17	4	.312	1	0	1	0	3	0	15
286			min	-170.191	3	.038	12	-.033	3	0	15	0	1	-.001	4
287		11	max	164.489	2	.017	2	.312	1	0	1	0	3	0	15
288			min	-170.243	3	-.051	3	-.033	3	0	15	0	1	-.001	4
289		12	max	164.42	2	-.043	15	.312	1	0	1	0	3	0	15
290			min	-170.295	3	-.184	4	-.033	3	0	15	0	1	-.001	4
291		13	max	164.351	2	-.085	15	.312	1	0	1	0	3	0	15
292			min	-170.347	3	-.361	4	-.033	3	0	15	0	1	-.001	4
293		14	max	164.282	2	-.126	15	.312	1	0	1	0	3	0	15
294			min	-170.399	3	-.537	4	-.033	3	0	15	0	1	-.001	4
295		15	max	164.212	2	-.168	15	.312	1	0	1	0	3	0	15
296			min	-170.451	3	-.714	4	-.033	3	0	15	0	2	0	4
297		16	max	164.143	2	-.209	15	.312	1	0	1	0	3	0	15
298			min	-170.503	3	-.891	4	-.033	3	0	15	0	10	0	4
299		17	max	164.074	2	-.251	15	.312	1	0	1	0	3	0	15
300			min	-170.555	3	-1.068	4	-.033	3	0	15	0	15	0	4
301		18	max	164.004	2	-.293	15	.312	1	0	1	0	3	0	15
302			min	-170.607	3	-1.245	4	-.033	3	0	15	0	15	0	4
303		19	max	163.935	2	-.334	15	.312	1	0	1	0	1	0	1
304			min	-170.659	3	-1.422	4	-.033	3	0	15	0	15	0	1
305	M12	1	max	331.889	1	0	1	1.841	1	0	1	0	2	0	1
306			min	3.89	12	0	1	.078	15	0	1	0	3	0	1
307		2	max	331.954	1	0	1	1.841	1	0	1	0	1	0	1
308			min	3.922	12	0	1	.078	15	0	1	0	15	0	1
309		3	max	332.019	1	0	1	1.841	1	0	1	0	1	0	1
310			min	3.955	12	0	1	.078	15	0	1	0	15	0	1
311		4	max	332.083	1	0	1	1.841	1	0	1	0	1	0	1
312			min	3.987	12	0	1	.078	15	0	1	0	15	0	1
313		5	max	332.148	1	0	1	1.841	1	0	1	0	1	0	1
314			min	4.019	12	0	1	.078	15	0	1	0	15	0	1
315		6	max	332.213	1	0	1	1.841	1	0	1	0	1	0	1
316			min	4.052	12	0	1	.078	15	0	1	0	15	0	1
317		7	max	332.277	1	0	1	1.841	1	0	1	.001	1	0	1
318			min	4.084	12	0	1	.078	15	0	1	0	15	0	1
319		8	max	332.342	1	0	1	1.841	1	0	1	.001	1	0	1
320			min	4.116	12	0	1	.078	15	0	1	0	15	0	1
321		9	max	332.407	1	0	1	1.841	1	0	1	.001	1	0	1
322			min	4.149	12	0	1	.078	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	332.472	1	0	1	1.841	1	0	1	.002	1	0	1
324		min	4.181	12	0	1	.078	15	0	1	0	15	0	1
325	11	max	332.536	1	0	1	1.841	1	0	1	.002	1	0	1
326		min	4.213	12	0	1	.078	15	0	1	0	15	0	1
327	12	max	332.601	1	0	1	1.841	1	0	1	.002	1	0	1
328		min	4.246	12	0	1	.078	15	0	1	0	15	0	1
329	13	max	332.666	1	0	1	1.841	1	0	1	.002	1	0	1
330		min	4.278	12	0	1	.078	15	0	1	0	15	0	1
331	14	max	332.73	1	0	1	1.841	1	0	1	.002	1	0	1
332		min	4.31	12	0	1	.078	15	0	1	0	15	0	1
333	15	max	332.795	1	0	1	1.841	1	0	1	.002	1	0	1
334		min	4.343	12	0	1	.078	15	0	1	0	15	0	1
335	16	max	332.86	1	0	1	1.841	1	0	1	.002	1	0	1
336		min	4.375	12	0	1	.078	15	0	1	0	15	0	1
337	17	max	332.925	1	0	1	1.841	1	0	1	.003	1	0	1
338		min	4.407	12	0	1	.078	15	0	1	0	15	0	1
339	18	max	332.989	1	0	1	1.841	1	0	1	.003	1	0	1
340		min	4.44	12	0	1	.078	15	0	1	0	15	0	1
341	19	max	333.054	1	0	1	1.841	1	0	1	.003	1	0	1
342		min	4.472	12	0	1	.078	15	0	1	0	15	0	1
343	M1	1	max	103.91	1	330.672	3	-1.551	15	0	.073	1	0	2
344		min	4.228	15	-232.168	1	-36.982	1	0	3	.003	15	0	3
345	2	max	104.05	1	330.49	3	-1.551	15	0	1	.065	1	.051	1
346		min	4.27	15	-232.41	1	-36.982	1	0	3	.003	15	-.072	3
347	3	max	85.587	3	5.349	9	-1.539	15	0	12	.056	1	.1	1
348		min	-12.036	10	-23.195	2	-36.863	1	0	1	.002	15	-.142	3
349	4	max	85.692	3	5.147	9	-1.539	15	0	12	.048	1	.104	2
350		min	-11.92	10	-23.437	2	-36.863	1	0	1	.002	15	-.139	3
351	5	max	85.797	3	4.946	9	-1.539	15	0	12	.04	1	.109	2
352		min	-11.803	10	-23.679	2	-36.863	1	0	1	.002	15	-.137	3
353	6	max	85.902	3	4.744	9	-1.539	15	0	12	.032	1	.115	2
354		min	-11.687	10	-23.921	2	-36.863	1	0	1	.001	15	-.134	3
355	7	max	86.006	3	4.543	9	-1.539	15	0	12	.024	1	.12	2
356		min	-11.571	10	-24.163	2	-36.863	1	0	1	.001	15	-.131	3
357	8	max	86.111	3	4.341	9	-1.539	15	0	12	.016	1	.125	2
358		min	-11.454	10	-24.404	2	-36.863	1	0	1	0	15	-.128	3
359	9	max	86.216	3	4.14	9	-1.539	15	0	12	.008	1	.13	2
360		min	-11.338	10	-24.646	2	-36.863	1	0	1	0	15	-.125	3
361	10	max	86.32	3	3.938	9	-1.539	15	0	12	.001	3	.136	2
362		min	-11.221	10	-24.888	2	-36.863	1	0	1	0	10	-.122	3
363	11	max	86.425	3	3.737	9	-1.539	15	0	12	0	3	.141	2
364		min	-11.105	10	-25.13	2	-36.863	1	0	1	-.008	1	-.119	3
365	12	max	86.53	3	3.535	9	-1.539	15	0	12	0	12	.147	2
366		min	-10.989	10	-25.372	2	-36.863	1	0	1	-.016	1	-.116	3
367	13	max	86.635	3	3.334	9	-1.539	15	0	12	0	15	.152	2
368		min	-10.872	10	-25.613	2	-36.863	1	0	1	-.024	1	-.113	3
369	14	max	86.739	3	3.132	9	-1.539	15	0	12	-.001	15	.158	2
370		min	-10.756	10	-25.855	2	-36.863	1	0	1	-.032	1	-.109	3
371	15	max	86.844	3	2.931	9	-1.539	15	0	12	-.002	15	.163	2
372		min	-10.64	10	-26.097	2	-36.863	1	0	1	-.04	1	-.106	3
373	16	max	88.314	2	110.777	2	-1.551	15	0	1	-.002	15	.168	2
374		min	-5.67	3	-157.712	3	-37.11	1	0	12	-.048	1	-.102	3
375	17	max	88.453	2	110.535	2	-1.551	15	0	1	-.002	15	.144	2
376		min	-5.565	3	-157.893	3	-37.11	1	0	12	-.056	1	-.067	3
377	18	max	-4.269	15	332.945	2	-1.587	15	0	3	-.003	15	.072	2
378		min	-104.035	1	-154.77	3	-38.033	1	0	2	-.064	1	-.034	3
379	19	max	-4.227	15	332.703	2	-1.587	15	0	3	-.003	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380			min	-103.896	1	-154.952	3	-38.033	1	0	2	-.073	1	0	3
381	M5	1	max	238.413	1	1080.667	3	0	10	0	1	.007	3	0	3
382			min	5.54	12	-757.675	1	-58.2	3	0	3	0	10	0	2
383		2	max	238.552	1	1080.485	3	0	10	0	1	0	1	.164	1
384			min	5.61	12	-757.917	1	-58.2	3	0	3	-.005	3	-.234	3
385		3	max	256.634	3	5.379	9	6.518	3	0	3	0	1	.325	1
386			min	-45.508	2	-83.75	2	-.526	1	0	1	-.017	3	-.463	3
387		4	max	256.739	3	5.178	9	6.518	3	0	3	0	1	.339	2
388			min	-45.369	2	-83.992	2	-.526	1	0	1	-.016	3	-.453	3
389		5	max	256.843	3	4.976	9	6.518	3	0	3	0	1	.358	2
390			min	-45.229	2	-84.234	2	-.526	1	0	1	-.014	3	-.442	3
391		6	max	256.948	3	4.775	9	6.518	3	0	3	0	1	.376	2
392			min	-45.09	2	-84.476	2	-.526	1	0	1	-.013	3	-.432	3
393		7	max	257.053	3	4.573	9	6.518	3	0	3	0	1	.394	2
394			min	-44.95	2	-84.718	2	-.526	1	0	1	-.012	3	-.421	3
395		8	max	257.158	3	4.372	9	6.518	3	0	3	0	1	.413	2
396			min	-44.81	2	-84.959	2	-.526	1	0	1	-.01	3	-.41	3
397		9	max	257.262	3	4.17	9	6.518	3	0	3	0	1	.431	2
398			min	-44.671	2	-85.201	2	-.526	1	0	1	-.009	3	-.4	3
399		10	max	257.367	3	3.969	9	6.518	3	0	3	0	2	.45	2
400			min	-44.531	2	-85.443	2	-.526	1	0	1	-.007	3	-.389	3
401		11	max	257.472	3	3.767	9	6.518	3	0	3	0	10	.468	2
402			min	-44.391	2	-85.685	2	-.526	1	0	1	-.006	3	-.378	3
403		12	max	257.576	3	3.565	9	6.518	3	0	3	0	10	.487	2
404			min	-44.252	2	-85.927	2	-.526	1	0	1	-.005	3	-.367	3
405		13	max	257.681	3	3.364	9	6.518	3	0	3	0	10	.505	2
406			min	-44.112	2	-86.168	2	-.526	1	0	1	-.003	3	-.357	3
407		14	max	257.786	3	3.162	9	6.518	3	0	3	0	10	.524	2
408			min	-43.973	2	-86.41	2	-.526	1	0	1	-.002	3	-.346	3
409		15	max	257.891	3	2.961	9	6.518	3	0	3	0	10	.543	2
410			min	-43.833	2	-86.652	2	-.526	1	0	1	0	1	-.335	3
411		16	max	287.402	2	419.363	2	6.491	3	0	3	0	3	.557	2
412			min	-22.528	3	-479.019	3	-.54	1	0	10	0	1	-.32	3
413		17	max	287.542	2	419.121	2	6.491	3	0	3	.002	3	.466	2
414			min	-22.424	3	-479.2	3	-.54	1	0	10	0	1	-.216	3
415		18	max	-7.506	12	1082.726	2	5.95	3	0	3	.003	3	.234	2
416			min	-238.575	1	-499.203	3	-.12	1	0	1	0	1	-.108	3
417		19	max	-7.436	12	1082.484	2	5.95	3	0	3	.005	3	0	3
418			min	-238.435	1	-499.385	3	-.12	1	0	1	0	1	0	2
419	M9	1	max	103.563	1	330.62	3	62.807	3	0	3	-.003	15	0	2
420			min	4.21	15	-232.167	1	1.585	15	0	1	-.072	1	0	3
421		2	max	103.702	1	330.439	3	62.807	3	0	3	-.001	12	.051	1
422			min	4.252	15	-232.408	1	1.585	15	0	1	-.064	1	-.072	3
423		3	max	85.473	3	5.326	9	36.031	1	0	1	.011	3	.1	1
424			min	-11.611	10	-23.202	2	-1.351	3	0	15	-.055	1	-.142	3
425		4	max	85.577	3	5.125	9	36.031	1	0	1	.011	3	.104	2
426			min	-11.495	10	-23.444	2	-1.351	3	0	15	-.047	1	-.139	3
427		5	max	85.682	3	4.923	9	36.031	1	0	1	.011	3	.109	2
428			min	-11.379	10	-23.686	2	-1.351	3	0	15	-.039	1	-.137	3
429		6	max	85.787	3	4.722	9	36.031	1	0	1	.01	3	.114	2
430			min	-11.262	10	-23.928	2	-1.351	3	0	15	-.031	1	-.134	3
431		7	max	85.892	3	4.52	9	36.031	1	0	1	.01	3	.12	2
432			min	-11.146	10	-24.17	2	-1.351	3	0	15	-.023	1	-.131	3
433		8	max	85.996	3	4.319	9	36.031	1	0	1	.01	3	.125	2
434			min	-11.03	10	-24.412	2	-1.351	3	0	15	-.016	1	-.128	3
435		9	max	86.101	3	4.117	9	36.031	1	0	1	.009	3	.13	2
436			min	-10.913	10	-24.653	2	-1.351	3	0	15	-.008	1	-.125	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	86.206	3	3.916	9	36.031	1	0	1	.009	3	.136	2
438		min	-10.797	10	-24.895	2	-1.351	3	0	15	0	1	-.122	3
439	11	max	86.31	3	3.714	9	36.031	1	0	1	.009	3	.141	2
440		min	-10.681	10	-25.137	2	-1.351	3	0	15	0	15	-.119	3
441	12	max	86.415	3	3.513	9	36.031	1	0	1	.016	1	.147	2
442		min	-10.564	10	-25.379	2	-1.351	3	0	15	0	15	-.116	3
443	13	max	86.52	3	3.311	9	36.031	1	0	1	.023	1	.152	2
444		min	-10.448	10	-25.621	2	-1.351	3	0	15	0	15	-.113	3
445	14	max	86.625	3	3.109	9	36.031	1	0	1	.031	1	.158	2
446		min	-10.332	10	-25.862	2	-1.351	3	0	15	.001	15	-.109	3
447	15	max	86.729	3	2.908	9	36.031	1	0	1	.039	1	.163	2
448		min	-10.215	10	-26.104	2	-1.351	3	0	15	.002	15	-.106	3
449	16	max	88.55	2	110.406	2	36.303	1	0	15	.047	1	.168	2
450		min	-6.138	3	-158.19	3	-1.38	3	0	1	.002	15	-.102	3
451	17	max	88.689	2	110.165	2	36.303	1	0	15	.055	1	.144	2
452		min	-6.033	3	-158.371	3	-1.38	3	0	1	.002	15	-.067	3
453	18	max	-4.252	15	332.945	2	38.163	1	0	2	.063	1	.072	2
454		min	-103.694	1	-154.764	3	-.918	3	0	3	.003	15	-.034	3
455	19	max	-4.21	15	332.703	2	38.163	1	0	2	.072	1	0	2
456		min	-103.554	1	-154.945	3	-.918	3	0	3	.003	15	0	3
457	M13	1	max	62.803	3	231.89	1	-4.21	15	0	.072	1	0	1
458		min	1.585	15	-330.641	3	-103.554	1	0	3	.003	15	0	3
459	2	max	62.803	3	164.165	1	-3.213	15	0	2	.019	1	.165	3
460		min	1.585	15	-233.874	3	-78.783	1	0	3	0	10	-.116	1
461	3	max	62.803	3	96.439	1	-2.216	15	0	2	.007	3	.273	3
462		min	1.585	15	-137.107	3	-54.011	1	0	3	-.02	1	-.192	1
463	4	max	62.803	3	28.733	2	-1.219	15	0	2	.003	3	.325	3
464		min	1.585	15	-40.341	3	-29.24	1	0	3	-.044	1	-.228	1
465	5	max	62.803	3	56.426	3	.648	10	0	2	.001	3	.32	3
466		min	1.585	15	-39.012	1	-4.468	1	0	3	-.054	1	-.225	1
467	6	max	62.803	3	153.192	3	20.303	1	0	2	0	3	.259	3
468		min	1.585	15	-106.737	1	-1.705	3	0	3	-.05	1	-.183	1
469	7	max	62.803	3	249.959	3	45.075	1	0	2	0	12	.141	3
470		min	1.585	15	-174.462	1	-.254	3	0	3	-.031	1	-.101	2
471	8	max	62.803	3	346.726	3	69.846	1	0	2	.004	2	.021	1
472		min	1.585	15	-242.188	1	.987	12	0	3	0	3	-.033	3
473	9	max	62.803	3	443.492	3	94.618	1	0	2	.051	1	.182	1
474		min	1.585	15	-309.913	1	1.954	12	0	3	0	12	-.263	3
475	10	max	62.803	3	540.259	3	119.389	1	0	2	.113	1	.383	1
476		min	1.585	15	-377.639	1	2.922	12	0	3	-.005	3	-.55	3
477	11	max	37.073	1	309.913	1	-1.644	12	0	3	.05	1	.182	1
478		min	1.551	15	-443.492	3	-94.27	1	0	2	-.006	3	-.263	3
479	12	max	37.073	1	242.188	1	-.676	12	0	3	.004	2	.021	1
480		min	1.551	15	-346.726	3	-69.498	1	0	2	-.007	3	-.033	3
481	13	max	37.073	1	174.462	1	.766	3	0	3	-.001	15	.141	3
482		min	1.551	15	-249.959	3	-44.727	1	0	2	-.031	1	-.101	2
483	14	max	37.073	1	106.737	1	2.217	3	0	3	-.002	15	.259	3
484		min	1.551	15	-153.192	3	-19.955	1	0	2	-.05	1	-.183	1
485	15	max	37.073	1	39.012	1	4.816	1	0	3	-.002	15	.32	3
486		min	1.551	15	-56.426	3	-.647	10	0	2	-.054	1	-.225	1
487	16	max	37.073	1	40.341	3	29.588	1	0	3	-.001	12	.325	3
488		min	1.551	15	-28.733	2	1.236	15	0	2	-.044	1	-.228	1
489	17	max	37.073	1	137.107	3	54.359	1	0	3	.001	3	.273	3
490		min	1.551	15	-96.439	1	2.233	15	0	2	-.02	1	-.192	1
491	18	max	37.073	1	233.874	3	79.13	1	0	3	.019	1	.165	3
492		min	1.551	15	-164.165	1	3.231	15	0	2	0	10	-.116	1
493	19	max	37.073	1	330.641	3	103.902	1	0	3	.073	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	1.551	15	-231.89	1	4.228	15	0	2	.003	15	0	3
495	M16	1	max	.921	3	332.856	2	-4.21	15	0	3	.072	1	0	2
496			min	-38.068	1	-154.97	3	-103.563	1	0	2	.003	15	0	3
497		2	max	.921	3	235.657	2	-3.213	15	0	3	.019	1	.077	3
498			min	-38.068	1	-110.002	3	-78.792	1	0	2	0	10	-.166	2
499		3	max	.921	3	138.458	2	-2.216	15	0	3	0	12	.128	3
500			min	-38.068	1	-65.033	3	-54.021	1	0	2	-.02	1	-.275	2
501		4	max	.921	3	41.259	2	-1.219	15	0	3	-.002	15	.153	3
502			min	-38.068	1	-20.065	3	-29.249	1	0	2	-.044	1	-.327	2
503		5	max	.921	3	24.903	3	.636	10	0	3	-.002	15	.152	3
504			min	-38.068	1	-55.941	2	-4.478	1	0	2	-.054	1	-.323	2
505		6	max	.921	3	69.872	3	20.294	1	0	3	-.002	15	.124	3
506			min	-38.068	1	-153.14	2	-.749	3	0	2	-.05	1	-.262	2
507		7	max	.921	3	114.84	3	45.065	1	0	3	-.001	15	.07	3
508			min	-38.068	1	-250.339	2	.593	12	0	2	-.031	1	-.144	2
509		8	max	.921	3	159.809	3	69.837	1	0	3	.004	2	.03	2
510			min	-38.068	1	-347.538	2	1.56	12	0	2	-.005	3	-.01	3
511		9	max	.921	3	204.777	3	94.608	1	0	3	.051	1	.261	2
512			min	-38.068	1	-444.738	2	2.528	12	0	2	-.003	3	-.116	3
513		10	max	-1.591	15	-9.636	15	119.38	1	0	15	.113	1	.549	2
514			min	-38.068	1	-541.937	2	-5.803	3	0	2	.003	12	-.249	3
515		11	max	-1.587	15	444.738	2	-2.974	12	0	2	.05	1	.261	2
516			min	-37.943	1	-204.777	3	-94.267	1	0	3	0	12	-.116	3
517		12	max	-1.587	15	347.538	2	-2.007	12	0	2	.004	2	.03	2
518			min	-37.943	1	-159.809	3	-69.496	1	0	3	0	3	-.01	3
519		13	max	-1.587	15	250.339	2	-1.039	12	0	2	-.001	15	.07	3
520			min	-37.943	1	-114.84	3	-44.724	1	0	3	-.031	1	-.144	2
521		14	max	-1.587	15	153.14	2	0	3	0	2	-.002	12	.124	3
522			min	-37.943	1	-69.872	3	-19.953	1	0	3	-.05	1	-.262	2
523		15	max	-1.587	15	55.941	2	4.819	1	0	2	-.002	12	.152	3
524			min	-37.943	1	-24.903	3	-.636	10	0	3	-.054	1	-.323	2
525		16	max	-1.587	15	20.065	3	29.59	1	0	2	0	12	.153	3
526			min	-37.943	1	-41.259	2	1.235	15	0	3	-.044	1	-.327	2
527		17	max	-1.587	15	65.033	3	54.362	1	0	2	.001	3	.128	3
528			min	-37.943	1	-138.458	2	2.232	15	0	3	-.02	1	-.275	2
529		18	max	-1.587	15	110.002	3	79.133	1	0	2	.019	1	.077	3
530			min	-37.943	1	-235.657	2	3.229	15	0	3	0	10	-.166	2
531		19	max	-1.587	15	154.97	3	103.905	1	0	2	.073	1	0	2
532			min	-37.943	1	-332.856	2	4.227	15	0	3	.003	15	0	3
533	M15	1	max	.851	13	1.851	4	.08	3	0	9	0	9	0	1
534			min	-76.083	3	0	1	-.023	9	0	3	0	3	0	1
535		2	max	.754	13	1.646	4	.08	3	0	9	0	9	0	1
536			min	-76.153	3	0	1	-.023	9	0	3	0	3	0	4
537		3	max	.657	13	1.44	4	.08	3	0	9	0	9	0	1
538			min	-76.224	3	0	1	-.023	9	0	3	0	3	-.001	4
539		4	max	.56	13	1.234	4	.08	3	0	9	0	9	0	1
540			min	-76.294	3	0	1	-.023	9	0	3	0	3	-.002	4
541		5	max	.463	13	1.029	4	.08	3	0	9	0	9	0	1
542			min	-76.365	3	0	1	-.023	9	0	3	0	3	-.002	4
543		6	max	.366	13	.823	4	.08	3	0	9	0	9	0	1
544			min	-76.435	3	0	1	-.023	9	0	3	0	3	-.002	4
545		7	max	.269	13	.617	4	.08	3	0	9	0	3	0	1
546			min	-76.506	3	0	1	-.023	9	0	3	0	9	-.003	4
547		8	max	.172	13	.411	4	.08	3	0	9	0	3	0	1
548			min	-76.576	3	0	1	-.023	9	0	3	0	9	-.003	4
549		9	max	.075	13	.206	4	.08	3	0	9	0	3	0	1
550			min	-76.647	3	0	1	-.023	9	0	3	0	9	-.003	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.08	3	0	9	0	3	0	1
552		min	-76.717	3	0	1	-.023	9	0	3	0	9	-.003	4
553	11	max	0	1	0	1	.08	3	0	9	0	3	0	1
554		min	-76.788	3	-.206	4	-.023	9	0	3	0	9	-.003	4
555	12	max	0	1	0	1	.08	3	0	9	0	3	0	1
556		min	-76.858	3	-.411	4	-.023	9	0	3	0	9	-.003	4
557	13	max	0	1	0	1	.08	3	0	9	0	3	0	1
558		min	-76.929	3	-.617	4	-.023	9	0	3	0	9	-.003	4
559	14	max	0	1	0	1	.08	3	0	9	0	3	0	1
560		min	-76.999	3	-.823	4	-.023	9	0	3	0	9	-.002	4
561	15	max	0	1	0	1	.08	3	0	9	0	3	0	1
562		min	-77.07	3	-1.029	4	-.023	9	0	3	0	9	-.002	4
563	16	max	0	1	0	1	.08	3	0	9	0	3	0	1
564		min	-77.14	3	-1.234	4	-.023	9	0	3	0	9	-.002	4
565	17	max	0	1	0	1	.08	3	0	9	0	3	0	1
566		min	-77.211	3	-1.44	4	-.023	9	0	3	0	9	-.001	4
567	18	max	0	1	0	1	.08	3	0	9	0	3	0	1
568		min	-77.281	3	-1.646	4	-.023	9	0	3	0	9	0	4
569	19	max	0	1	0	1	.08	3	0	9	0	3	0	1
570		min	-77.352	3	-1.851	4	-.023	9	0	3	0	9	0	1
571	M16A	1	max	0	10	1.851	.03	1	0	3	0	3	0	1
572		min	-76.311	3	0	10	-.033	3	0	2	0	1	0	1
573	2	max	0	10	1.646	4	.03	1	0	3	0	3	0	10
574		min	-76.24	3	0	10	-.033	3	0	2	0	1	0	4
575	3	max	0	10	1.44	4	.03	1	0	3	0	3	0	10
576		min	-76.17	3	0	10	-.033	3	0	2	0	1	-.001	4
577	4	max	0	10	1.234	4	.03	1	0	3	0	3	0	10
578		min	-76.099	3	0	10	-.033	3	0	2	0	1	-.002	4
579	5	max	0	10	1.029	4	.03	1	0	3	0	3	0	10
580		min	-76.029	3	0	10	-.033	3	0	2	0	1	-.002	4
581	6	max	0	10	.823	4	.03	1	0	3	0	3	0	10
582		min	-75.958	3	0	10	-.033	3	0	2	0	1	-.002	4
583	7	max	0	10	.617	4	.03	1	0	3	0	3	0	10
584		min	-75.888	3	0	10	-.033	3	0	2	0	1	-.003	4
585	8	max	0	10	.411	4	.03	1	0	3	0	3	0	10
586		min	-75.817	3	0	10	-.033	3	0	2	0	1	-.003	4
587	9	max	0	10	.206	4	.03	1	0	3	0	3	0	10
588		min	-75.747	3	0	10	-.033	3	0	2	0	1	-.003	4
589	10	max	0	10	0	1	.03	1	0	3	0	3	0	10
590		min	-75.676	3	0	1	-.033	3	0	2	0	1	-.003	4
591	11	max	0	10	0	10	.03	1	0	3	0	3	0	10
592		min	-75.606	3	-.206	4	-.033	3	0	2	0	1	-.003	4
593	12	max	.089	2	0	10	.03	1	0	3	0	3	0	10
594		min	-75.536	3	-.411	4	-.033	3	0	2	0	1	-.003	4
595	13	max	.183	2	0	10	.03	1	0	3	0	2	0	10
596		min	-75.465	3	-.617	4	-.033	3	0	2	0	4	-.003	4
597	14	max	.277	2	0	10	.03	1	0	3	0	2	0	10
598		min	-75.395	3	-.823	4	-.033	3	0	2	0	3	-.002	4
599	15	max	.371	2	0	10	.03	1	0	3	0	2	0	10
600		min	-75.324	3	-1.029	4	-.033	3	0	2	0	3	-.002	4
601	16	max	.465	2	0	10	.03	1	0	3	0	2	0	10
602		min	-75.254	3	-1.234	4	-.033	3	0	2	0	3	-.002	4
603	17	max	.559	2	0	10	.03	1	0	3	0	2	0	10
604		min	-75.183	3	-1.44	4	-.033	3	0	2	0	3	-.001	4
605	18	max	.653	2	0	10	.03	1	0	3	0	2	0	10
606		min	-75.113	3	-1.646	4	-.033	3	0	2	0	3	0	4
607	19	max	.747	2	0	10	.03	1	0	3	0	2	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-75.042	3	-1.851	4	-.033	3	0	2	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.009	2	.007	1	-2.422e-5	15	NC	3	NC	2	
2			min	-.003	3	-.009	3	-.002	3	-5.833e-4	1	4374.501	2	5441.972	1	
3			2	max	.002	1	.008	2	.007	1	-2.318e-5	15	NC	3	NC	2
4				min	-.003	3	-.009	3	-.001	3	-5.581e-4	1	4779.331	2	5862.31	1
5			3	max	.002	1	.007	2	.006	1	-2.214e-5	15	NC	1	NC	2
6				min	-.003	3	-.008	3	-.001	3	-5.33e-4	1	5261.805	2	6359.016	1
7			4	max	.002	1	.007	2	.006	1	-2.11e-5	15	NC	1	NC	2
8				min	-.003	3	-.008	3	-.001	3	-5.079e-4	1	5840.906	2	6950.346	1
9			5	max	.002	1	.006	2	.005	1	-2.006e-5	15	NC	1	NC	2
10				min	-.003	3	-.008	3	-.001	3	-4.827e-4	1	6542.014	2	7660.691	1
11		6	max	.002	1	.005	2	.005	1	-1.903e-5	15	NC	1	NC	2	
12			min	-.002	3	-.007	3	0	3	-4.576e-4	1	7399.625	2	8523.206	1	
13		7	max	.002	1	.005	2	.004	1	-1.799e-5	15	NC	1	NC	2	
14			min	-.002	3	-.007	3	0	3	-4.324e-4	1	8461.538	2	9583.901	1	
15		8	max	.001	1	.004	2	.004	1	-1.695e-5	15	NC	1	NC	1	
16			min	-.002	3	-.006	3	0	3	-4.073e-4	1	9795.482	2	NC	1	
17		9	max	.001	1	.003	2	.003	1	-1.591e-5	15	NC	1	NC	1	
18			min	-.002	3	-.006	3	0	3	-3.821e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.003	1	-1.487e-5	15	NC	1	NC	1	
20			min	-.002	3	-.005	3	0	3	-3.57e-4	1	NC	1	NC	1	
21		11	max	.001	1	.002	2	.002	1	-1.384e-5	15	NC	1	NC	1	
22			min	-.002	3	-.005	3	0	3	-3.318e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	-1.28e-5	15	NC	1	NC	1	
24			min	-.001	3	-.004	3	0	3	-3.067e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	.001	1	-1.176e-5	15	NC	1	NC	1	
26			min	-.001	3	-.004	3	0	3	-2.815e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.001	1	-1.072e-5	15	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-2.564e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-9.683e-6	15	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-2.312e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-8.645e-6	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-2.061e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-7.607e-6	15	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-1.809e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-6.569e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	3	-1.558e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-5.531e-6	15	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.306e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	6.163e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	2.608e-6	15	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	7.601e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	3.192e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	9.038e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	3.775e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	1.048e-4	1	NC	1	NC	1
46				min	0	2	-.003	3	0	1	4.358e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	1.191e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	4.942e-6	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	1.335e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	0	1	5.525e-6	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	1.479e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	1	6.108e-6	15	NC	1	NC	1
53	8	max	0	3	0	2	0	3	1.622e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	6.692e-6	15	NC	1	NC	1
55	9	max	0	3	.001	2	0	2	1.766e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	9	7.275e-6	15	NC	1	NC	1
57	10	max	0	3	.002	2	0	2	1.91e-4	1	NC	1	NC	1
58		min	0	2	-.007	3	0	15	7.858e-6	15	NC	1	NC	1
59	11	max	.001	3	.002	2	0	1	2.054e-4	1	NC	1	NC	1
60		min	-.001	2	-.007	3	0	15	8.442e-6	15	NC	1	NC	1
61	12	max	.001	3	.003	2	0	1	2.197e-4	1	NC	1	NC	1
62		min	-.001	2	-.007	3	0	15	9.025e-6	15	NC	1	NC	1
63	13	max	.001	3	.003	2	.001	1	2.341e-4	1	NC	1	NC	1
64		min	-.001	2	-.008	3	0	15	9.608e-6	15	NC	1	NC	1
65	14	max	.001	3	.004	2	.002	1	2.485e-4	1	NC	1	NC	1
66		min	-.001	2	-.008	3	0	15	1.019e-5	15	NC	1	NC	1
67	15	max	.001	3	.005	2	.002	1	2.629e-4	1	NC	1	NC	1
68		min	-.001	2	-.008	3	0	15	1.078e-5	15	9507.345	2	NC	1
69	16	max	.002	3	.006	2	.002	1	2.772e-4	1	NC	1	NC	1
70		min	-.002	2	-.008	3	0	15	1.136e-5	15	8007.085	2	NC	1
71	17	max	.002	3	.007	2	.003	1	2.916e-4	1	NC	1	NC	1
72		min	-.002	2	-.008	3	0	15	1.194e-5	15	6857.193	2	NC	1
73	18	max	.002	3	.008	2	.003	1	3.06e-4	1	NC	1	NC	1
74		min	-.002	2	-.008	3	0	15	1.252e-5	15	5964.391	2	NC	1
75	19	max	.002	3	.009	2	.004	1	3.204e-4	1	NC	3	NC	1
76		min	-.002	2	-.008	3	0	15	1.311e-5	15	5264.152	2	NC	1
77	M4	1	max	.002	1	.01	2	15	-1.891e-5	12	NC	1	NC	2
78		min	0	12	-.009	3	-.003	1	-4.665e-4	1	NC	1	6959.554	1
79	2	max	.001	1	.01	2	0	15	-1.891e-5	12	NC	1	NC	2
80		min	0	12	-.008	3	-.003	1	-4.665e-4	1	NC	1	7592.463	1
81	3	max	.001	1	.009	2	0	15	-1.891e-5	12	NC	1	NC	2
82		min	0	12	-.008	3	-.002	1	-4.665e-4	1	NC	1	8345.715	1
83	4	max	.001	1	.009	2	0	15	-1.891e-5	12	NC	1	NC	2
84		min	0	12	-.007	3	-.002	1	-4.665e-4	1	NC	1	9251.055	1
85	5	max	.001	1	.008	2	0	15	-1.891e-5	12	NC	1	NC	1
86		min	0	12	-.007	3	-.002	1	-4.665e-4	1	NC	1	NC	1
87	6	max	.001	1	.007	2	0	15	-1.891e-5	12	NC	1	NC	1
88		min	0	12	-.006	3	-.002	1	-4.665e-4	1	NC	1	NC	1
89	7	max	.001	1	.007	2	0	15	-1.891e-5	12	NC	1	NC	1
90		min	0	12	-.006	3	-.001	1	-4.665e-4	1	NC	1	NC	1
91	8	max	0	1	.006	2	0	15	-1.891e-5	12	NC	1	NC	1
92		min	0	12	-.005	3	-.001	1	-4.665e-4	1	NC	1	NC	1
93	9	max	0	1	.006	2	0	15	-1.891e-5	12	NC	1	NC	1
94		min	0	12	-.005	3	-.001	1	-4.665e-4	1	NC	1	NC	1
95	10	max	0	1	.005	2	0	15	-1.891e-5	12	NC	1	NC	1
96		min	0	12	-.004	3	0	1	-4.665e-4	1	NC	1	NC	1
97	11	max	0	1	.005	2	0	15	-1.891e-5	12	NC	1	NC	1
98		min	0	12	-.004	3	0	1	-4.665e-4	1	NC	1	NC	1
99	12	max	0	1	.004	2	0	15	-1.891e-5	12	NC	1	NC	1
100		min	0	12	-.003	3	0	1	-4.665e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-1.891e-5	12	NC	1	NC	1
102		min	0	12	-.003	3	0	1	-4.665e-4	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-1.891e-5	12	NC	1	NC	1
104		min	0	12	-.002	3	0	1	-4.665e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-1.891e-5	12	NC	1	NC	1
106		min	0	12	-.002	3	0	1	-4.665e-4	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-1.891e-5	12	NC	1	NC	1
108		min	0	12	-.001	3	0	1	-4.665e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-1.891e-5	12	NC	1	NC	1
110			min	0	12	0	3	0	1	-4.665e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.891e-5	12	NC	1	NC	1
112			min	0	12	0	3	0	1	-4.665e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.891e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-4.665e-4	1	NC	1	NC	1
115	M6	1	max	.007	1	.031	2	.003	1	3.929e-4	3	NC	3	NC	1
116			min	-.011	3	-.028	3	-.005	3	-6.264e-8	10	1271.928	2	7895.299	3
117		2	max	.007	1	.029	2	.003	1	3.806e-4	3	NC	3	NC	1
118			min	-.01	3	-.027	3	-.005	3	-9.945e-7	2	1361.211	2	8387.571	3
119		3	max	.006	1	.027	2	.002	1	3.683e-4	3	NC	3	NC	1
120			min	-.01	3	-.025	3	-.004	3	-2.106e-6	2	1463.546	2	8971.34	3
121		4	max	.006	1	.025	2	.002	1	3.561e-4	3	NC	3	NC	1
122			min	-.009	3	-.024	3	-.004	3	-4.196e-6	1	1581.547	2	9666.194	3
123		5	max	.006	1	.023	2	.002	1	3.438e-4	3	NC	3	NC	1
124			min	-.008	3	-.022	3	-.004	3	-7.78e-6	1	1718.58	2	NC	1
125		6	max	.005	1	.021	2	.002	1	3.316e-4	3	NC	3	NC	1
126			min	-.008	3	-.021	3	-.003	3	-1.136e-5	1	1879.056	2	NC	1
127		7	max	.005	1	.019	2	.002	1	3.193e-4	3	NC	3	NC	1
128			min	-.007	3	-.019	3	-.003	3	-1.495e-5	1	2068.858	2	NC	1
129		8	max	.004	1	.017	2	.001	1	3.07e-4	3	NC	3	NC	1
130			min	-.007	3	-.018	3	-.003	3	-1.853e-5	1	2296.012	2	NC	1
131		9	max	.004	1	.015	2	.001	1	2.948e-4	3	NC	3	NC	1
132			min	-.006	3	-.016	3	-.002	3	-2.212e-5	1	2571.755	2	NC	1
133		10	max	.004	1	.014	2	.001	1	2.825e-4	3	NC	3	NC	1
134			min	-.005	3	-.015	3	-.002	3	-2.57e-5	1	2912.32	2	NC	1
135		11	max	.003	1	.012	2	0	1	2.703e-4	3	NC	3	NC	1
136			min	-.005	3	-.013	3	-.002	3	-2.928e-5	1	3342.046	2	NC	1
137		12	max	.003	1	.01	2	0	1	2.58e-4	3	NC	3	NC	1
138			min	-.004	3	-.012	3	-.002	3	-3.287e-5	1	3899.175	2	NC	1
139		13	max	.002	1	.008	2	0	1	2.457e-4	3	NC	3	NC	1
140			min	-.004	3	-.01	3	-.001	3	-3.645e-5	1	4647.419	2	NC	1
141		14	max	.002	1	.007	2	0	1	2.335e-4	3	NC	3	NC	1
142			min	-.003	3	-.008	3	0	3	-4.004e-5	1	5701.433	2	NC	1
143		15	max	.002	1	.005	2	0	1	2.212e-4	3	NC	1	NC	1
144			min	-.002	3	-.007	3	0	3	-4.362e-5	1	7290.474	2	NC	1
145		16	max	.001	1	.004	2	0	1	2.09e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-4.721e-5	1	9949.387	2	NC	1
147		17	max	0	1	.003	2	0	1	1.967e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-5.079e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.844e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-5.437e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.722e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-5.796e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.712e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-8.08e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	2.31e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.013e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.909e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-3.945e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	1.507e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.878e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.001	3	1.106e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	0	10	8574.094	2	NC	1
163		6	max	.002	3	.007	2	.002	3	2.257e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	0	10	6870.991	2	NC	1
165		7	max	.002	3	.008	2	.002	3	4.325e-5	3	NC	3	NC	1



***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	1	-9.802e-8	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.227e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-9.802e-8	10	NC	1	NC	1
226			min	0	3	-.002	3	0	3	-2.227e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-9.802e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.227e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.009	2	0	3	5.893e-4	1	NC	3	NC	1
230			min	-.003	3	-.009	3	-.001	1	-4.475e-4	3	4380.024	2	NC	1
231		2	max	.002	1	.008	2	0	3	5.59e-4	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.001	1	-4.324e-4	3	4785.548	2	NC	1
233		3	max	.002	1	.007	2	0	3	5.287e-4	1	NC	1	NC	1
234			min	-.003	3	-.008	3	-.001	1	-4.173e-4	3	5268.885	2	NC	1
235		4	max	.002	1	.007	2	0	3	4.984e-4	1	NC	1	NC	1
236			min	-.003	3	-.008	3	-.001	1	-4.022e-4	3	5849.07	2	NC	1
237		5	max	.002	1	.006	2	0	3	4.681e-4	1	NC	1	NC	1
238			min	-.002	3	-.008	3	-.001	1	-3.872e-4	3	6551.554	2	NC	1
239		6	max	.002	1	.005	2	0	3	4.378e-4	1	NC	1	NC	1
240			min	-.002	3	-.007	3	-.001	1	-3.721e-4	3	7410.935	2	NC	1
241		7	max	.002	1	.005	2	0	3	4.076e-4	1	NC	1	NC	1
242			min	-.002	3	-.007	3	-.001	1	-3.57e-4	3	8475.158	2	NC	1
243		8	max	.001	1	.004	2	0	3	3.773e-4	1	NC	1	NC	1
244			min	-.002	3	-.006	3	0	1	-3.42e-4	3	9812.172	2	NC	1
245		9	max	.001	1	.003	2	0	3	3.47e-4	1	NC	1	NC	1
246			min	-.002	3	-.006	3	0	1	-3.269e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	3.167e-4	1	NC	1	NC	1
248			min	-.002	3	-.006	3	0	1	-3.118e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	2.864e-4	1	NC	1	NC	1
250			min	-.001	3	-.005	3	0	1	-2.967e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	2.561e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-2.817e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	2.258e-4	1	NC	1	NC	1
254			min	-.001	3	-.004	3	0	1	-2.666e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.956e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-2.515e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.653e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.364e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.35e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.214e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.047e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.063e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	7.442e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.912e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	4.413e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.761e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.312e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.131e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	6.232e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-4.403e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	4.152e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.675e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	2.072e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-8.946e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	-7.703e-8	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-1.122e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-5.991e-6	15	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-1.349e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-6.952e-6	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-1.576e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	-7.913e-6	15	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-1.803e-4	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	-8.874e-6	15	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-2.03e-4	1	NC	1	NC	1
285		10	max	0	3	.002	2	0	15	-9.835e-6	15	NC	1	NC	1
286			min	0	2	-.007	3	-.002	3	-2.258e-4	1	NC	1	NC	1
287		11	max	.001	3	.002	2	0	15	-1.08e-5	15	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	1	-2.485e-4	1	NC	1	NC	1
289		12	max	.001	3	.003	2	0	15	-1.176e-5	15	NC	1	NC	1
290			min	-.001	2	-.007	3	-.003	1	-2.712e-4	1	NC	1	NC	1
291		13	max	.001	3	.003	2	0	15	-1.272e-5	15	NC	1	NC	1
292			min	-.001	2	-.008	3	-.004	1	-2.939e-4	1	NC	1	NC	1
293		14	max	.001	3	.004	2	0	15	-1.368e-5	15	NC	1	NC	1
294			min	-.001	2	-.008	3	-.004	1	-3.166e-4	1	NC	1	NC	1
295		15	max	.002	3	.005	2	0	15	-1.464e-5	15	NC	1	NC	2
296			min	-.001	2	-.008	3	-.005	1	-3.394e-4	1	9525.505	2	9653.768	1
297		16	max	.002	3	.006	2	0	15	-1.56e-5	15	NC	1	NC	2
298			min	-.002	2	-.008	3	-.005	1	-3.621e-4	1	8020.847	2	8633.802	1
299		17	max	.002	3	.007	2	0	15	-1.656e-5	15	NC	1	NC	2
300			min	-.002	2	-.008	3	-.006	1	-3.848e-4	1	6867.931	2	7817.154	1
301		18	max	.002	3	.008	2	0	15	-1.752e-5	15	NC	1	NC	2
302			min	-.002	2	-.008	3	-.006	1	-4.075e-4	1	5973.003	2	7156.164	1
303		19	max	.002	3	.009	2	0	15	-1.849e-5	15	NC	3	NC	2
304			min	-.002	2	-.008	3	-.007	1	-4.302e-4	1	5271.239	2	6617.05	1
305	M12	1	max	.002	1	.01	2	.006	1	3.976e-4	1	NC	1	NC	2
306			min	0	12	-.009	3	0	15	1.677e-5	15	NC	1	3290.756	1
307		2	max	.001	1	.01	2	.005	1	3.976e-4	1	NC	1	NC	2
308			min	0	12	-.009	3	0	15	1.677e-5	15	NC	1	3588.874	1
309		3	max	.001	1	.009	2	.005	1	3.976e-4	1	NC	1	NC	2
310			min	0	12	-.008	3	0	15	1.677e-5	15	NC	1	3943.732	1
311		4	max	.001	1	.009	2	.004	1	3.976e-4	1	NC	1	NC	2
312			min	0	12	-.008	3	0	15	1.677e-5	15	NC	1	4370.285	1
313		5	max	.001	1	.008	2	.004	1	3.976e-4	1	NC	1	NC	2
314			min	0	12	-.007	3	0	15	1.677e-5	15	NC	1	4888.91	1
315		6	max	.001	1	.007	2	.003	1	3.976e-4	1	NC	1	NC	2
316			min	0	12	-.007	3	0	15	1.677e-5	15	NC	1	5527.946	1
317		7	max	.001	1	.007	2	.003	1	3.976e-4	1	NC	1	NC	2
318			min	0	12	-.006	3	0	15	1.677e-5	15	NC	1	6327.761	1
319		8	max	0	1	.006	2	.003	1	3.976e-4	1	NC	1	NC	2
320			min	0	12	-.006	3	0	15	1.677e-5	15	NC	1	7347.468	1
321		9	max	0	1	.006	2	.002	1	3.976e-4	1	NC	1	NC	2
322			min	0	12	-.005	3	0	15	1.677e-5	15	NC	1	8676.488	1
323		10	max	0	1	.005	2	.002	1	3.976e-4	1	NC	1	NC	1
324			min	0	12	-.005	3	0	15	1.677e-5	15	NC	1	NC	1
325		11	max	0	1	.005	2	.001	1	3.976e-4	1	NC	1	NC	1
326			min	0	12	-.004	3	0	15	1.677e-5	15	NC	1	NC	1
327		12	max	0	1	.004	2	.001	1	3.976e-4	1	NC	1	NC	1
328			min	0	12	-.004	3	0	15	1.677e-5	15	NC	1	NC	1
329		13	max	0	1	.003	2	0	1	3.976e-4	1	NC	1	NC	1
330			min	0	12	-.003	3	0	15	1.677e-5	15	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	3.976e-4	1	NC	1	NC	1
332			min	0	12	-.003	3	0	15	1.677e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.976e-4	1	NC	1	NC	1
334			min	0	12	-.002	3	0	15	1.677e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	3.976e-4	1	NC	1	NC	1
336			min	0	12	-.002	3	0	15	1.677e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.976e-4	1	NC	1	NC	1
338			min	0	12	-.001	3	0	15	1.677e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.976e-4	1	NC	1	NC	1
340			min	0	12	0	3	0	15	1.677e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.976e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.677e-5	15	NC	1	NC	1
343	M1	1	max	.008	3	.024	3	.003	3	9.345e-3	2	NC	1	NC	1
344			min	-.008	2	-.021	2	-.003	1	-1.312e-2	3	NC	1	NC	1
345		2	max	.008	3	.014	3	.002	3	4.582e-3	2	NC	4	NC	1
346			min	-.008	2	-.012	2	-.006	1	-6.483e-3	3	4897.869	3	NC	1
347		3	max	.008	3	.005	3	.002	3	2.839e-5	3	NC	4	NC	1
348			min	-.008	2	-.004	2	-.007	1	-3.541e-4	1	2537.459	3	NC	1
349		4	max	.008	3	.004	2	.001	3	3.06e-5	3	NC	4	NC	2
350			min	-.008	2	-.003	3	-.008	1	-3.035e-4	1	1808.585	3	8371.444	1
351		5	max	.008	3	.01	2	0	3	3.281e-5	3	NC	4	NC	2
352			min	-.008	2	-.009	3	-.008	1	-2.528e-4	1	1451.551	2	8064.196	1
353		6	max	.008	3	.015	2	0	3	3.502e-5	3	NC	4	NC	2
354			min	-.008	2	-.014	3	-.008	1	-2.022e-4	1	1232.757	2	8669.671	1
355		7	max	.008	3	.02	2	0	3	3.723e-5	3	NC	5	NC	1
356			min	-.008	2	-.018	3	-.007	1	-1.515e-4	1	1097.694	2	NC	1
357		8	max	.008	3	.023	2	0	3	3.944e-5	3	NC	5	NC	1
358			min	-.008	2	-.021	3	-.006	1	-1.009e-4	1	1012.833	2	NC	1
359		9	max	.008	3	.025	2	0	3	4.165e-5	3	NC	5	NC	1
360			min	-.008	2	-.022	3	-.004	1	-5.021e-5	1	962.332	2	NC	1
361		10	max	.008	3	.026	2	0	3	4.386e-5	3	NC	5	NC	1
362			min	-.008	2	-.023	3	-.002	1	-4.715e-6	9	938.618	2	NC	1
363		11	max	.008	3	.025	2	0	3	5.109e-5	1	NC	5	NC	1
364			min	-.008	2	-.022	3	0	1	1.881e-6	15	938.933	2	NC	1
365		12	max	.008	3	.024	2	.001	1	1.017e-4	1	NC	5	NC	1
366			min	-.008	2	-.02	3	0	15	4.013e-6	15	964.315	2	NC	1
367		13	max	.008	3	.021	2	.002	1	1.524e-4	1	NC	4	NC	1
368			min	-.008	2	-.017	3	0	15	6.145e-6	15	1020.175	2	NC	1
369		14	max	.008	3	.016	2	.003	1	2.03e-4	1	NC	4	NC	2
370			min	-.008	2	-.013	3	0	15	8.277e-6	15	1119.059	2	8841.282	1
371		15	max	.008	3	.01	2	.004	1	2.537e-4	1	NC	4	NC	2
372			min	-.008	2	-.008	3	0	15	1.041e-5	15	1288.552	2	8190.299	1
373		16	max	.008	3	.003	2	.004	1	2.89e-4	1	NC	4	NC	2
374			min	-.008	2	-.003	3	0	15	1.19e-5	15	1596.249	2	8474.505	1
375		17	max	.008	3	.004	3	.003	1	4.652e-5	3	NC	4	NC	1
376			min	-.008	2	-.006	2	0	15	-4.088e-5	1	2259.343	2	NC	1
377		18	max	.008	3	.012	3	.001	1	6.655e-3	2	NC	4	NC	1
378			min	-.008	2	-.016	2	0	15	-3.21e-3	3	4377.435	2	NC	1
379		19	max	.008	3	.02	3	0	3	1.342e-2	2	NC	1	NC	1
380			min	-.008	2	-.028	2	-.002	1	-6.527e-3	3	NC	1	NC	1
381	M5	1	max	.025	3	.077	3	.003	3	3.733e-6	3	NC	1	NC	1
382			min	-.029	2	-.069	2	-.003	1	0	2	NC	1	NC	1
383		2	max	.025	3	.046	3	.004	3	1.087e-4	3	NC	4	NC	1
384			min	-.029	2	-.04	2	-.003	1	-4.975e-5	1	1521.403	3	NC	1
385		3	max	.025	3	.016	3	.005	3	2.117e-4	3	NC	5	NC	1
386			min	-.029	2	-.013	2	-.003	1	-9.865e-5	1	788.479	3	NC	1
387		4	max	.025	3	.011	2	.006	3	2.048e-4	3	NC	5	NC	1
388			min	-.029	2	-.009	3	-.003	1	-9.4e-5	1	557.036	2	NC	1
389		5	max	.025	3	.032	2	.006	3	1.98e-4	3	NC	5	NC	1
390			min	-.029	2	-.029	3	-.003	1	-8.934e-5	1	440.466	2	NC	1
391		6	max	.025	3	.05	2	.007	3	1.912e-4	3	NC	5	NC	1
392			min	-.029	2	-.046	3	-.003	1	-8.469e-5	1	373.805	2	NC	1
393		7	max	.025	3	.064	2	.007	3	1.844e-4	3	NC	5	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.008	3	.004	3	0	15	5.639e-5	3	NC	4	NC	2
452			min	-.008	2	-.006	2	-.006	1	-1.972e-4	1	2260.545	2	9855.07	1
453		18	max	.008	3	.012	3	0	15	3.262e-3	3	NC	4	NC	1
454			min	-.008	2	-.016	2	-.004	1	-6.666e-3	2	4379.691	2	NC	1
455		19	max	.008	3	.02	3	0	3	6.525e-3	3	NC	1	NC	1
456			min	-.008	2	-.028	2	-.001	1	-1.342e-2	2	NC	1	NC	1
457	M13	1	max	.003	1	.024	3	.008	3	3.701e-3	3	NC	1	NC	1
458			min	-.002	3	-.021	2	-.008	2	-3.337e-3	2	NC	1	NC	1
459		2	max	.003	1	.112	3	.01	1	4.619e-3	3	NC	4	NC	2
460			min	-.002	3	-.084	2	-.004	10	-4.185e-3	2	1419.585	3	8667.75	1
461		3	max	.003	1	.186	3	.029	1	5.537e-3	3	NC	5	NC	2
462			min	-.002	3	-.137	2	-.003	10	-5.032e-3	2	777.29	3	3812.011	1
463		4	max	.003	1	.233	3	.044	1	6.455e-3	3	NC	5	NC	3
464			min	-.002	3	-.171	2	-.003	10	-5.88e-3	2	601.142	3	2638.853	1
465		5	max	.003	1	.25	3	.049	1	7.373e-3	3	NC	5	NC	3
466			min	-.003	3	-.184	2	-.004	10	-6.728e-3	2	556.864	3	2345.102	1
467		6	max	.003	1	.236	3	.045	1	8.29e-3	3	NC	5	NC	2
468			min	-.003	3	-.176	2	-.006	10	-7.576e-3	2	592.793	3	2570.595	1
469		7	max	.003	1	.198	3	.03	1	9.208e-3	3	NC	5	NC	2
470			min	-.003	3	-.151	2	-.009	10	-8.424e-3	2	722.68	3	3648.046	1
471		8	max	.003	1	.147	3	.02	3	1.013e-2	3	NC	5	NC	2
472			min	-.003	3	-.116	2	-.016	2	-9.272e-3	2	1024.431	3	8768.464	9
473		9	max	.003	1	.099	3	.023	3	1.104e-2	3	NC	4	NC	1
474			min	-.003	3	-.084	2	-.025	2	-1.012e-2	2	1674.197	3	7724.052	2
475		10	max	.003	1	.077	3	.025	3	1.196e-2	3	NC	4	NC	4
476			min	-.003	3	-.069	2	-.029	2	-1.097e-2	2	2361.285	3	6156.608	2
477		11	max	.003	1	.099	3	.028	3	1.105e-2	3	NC	4	NC	1
478			min	-.003	3	-.084	2	-.025	2	-1.012e-2	2	1674.195	3	6460.015	3
479		12	max	.003	1	.147	3	.029	3	1.013e-2	3	NC	5	NC	2
480			min	-.003	3	-.116	2	-.015	2	-9.272e-3	2	1024.43	3	6117.822	3
481		13	max	.003	1	.198	3	.03	1	9.212e-3	3	NC	5	NC	2
482			min	-.003	3	-.151	2	-.009	10	-8.424e-3	2	722.679	3	3630.248	1
483		14	max	.003	1	.236	3	.045	1	8.296e-3	3	NC	5	NC	2
484			min	-.003	3	-.176	2	-.006	10	-7.577e-3	2	592.793	3	2567.875	1
485		15	max	.003	1	.25	3	.049	1	7.38e-3	3	NC	5	NC	5
486			min	-.003	3	-.184	2	-.004	10	-6.729e-3	2	556.863	3	2348.787	1
487		16	max	.003	1	.234	3	.043	1	6.463e-3	3	NC	5	NC	3
488			min	-.003	3	-.171	2	-.003	10	-5.882e-3	2	601.142	3	2649.868	1
489		17	max	.003	1	.186	3	.029	1	5.547e-3	3	NC	5	NC	2
490			min	-.003	3	-.137	2	-.003	10	-5.034e-3	2	777.289	3	3841.031	1
491		18	max	.003	1	.113	3	.011	3	4.63e-3	3	NC	4	NC	2
492			min	-.003	3	-.084	2	-.004	10	-4.186e-3	2	1419.585	3	8786.945	1
493		19	max	.003	1	.024	3	.008	3	3.714e-3	3	NC	1	NC	1
494			min	-.003	3	-.021	2	-.008	2	-3.339e-3	2	NC	1	NC	1
495	M16	1	max	.001	1	.02	3	.008	3	4.195e-3	2	NC	1	NC	1
496			min	0	3	-.028	2	-.008	2	-2.991e-3	3	NC	1	NC	1
497		2	max	.001	1	.064	3	.011	3	5.269e-3	2	NC	4	NC	2
498			min	0	3	-.118	2	-.004	10	-3.713e-3	3	1387.222	2	8676.481	1
499		3	max	.001	1	.101	3	.029	1	6.344e-3	2	NC	5	NC	2
500			min	0	3	-.194	2	-.003	10	-4.434e-3	3	758.429	2	3815.125	1
501		4	max	.001	1	.126	3	.043	1	7.418e-3	2	NC	5	NC	3
502			min	0	3	-.243	2	-.003	10	-5.155e-3	3	584.975	2	2640.963	1
503		5	max	.001	1	.136	3	.049	1	8.492e-3	2	NC	5	NC	5
504			min	0	3	-.261	2	-.004	10	-5.876e-3	3	539.476	2	2347.253	1
505		6	max	.001	1	.132	3	.044	1	9.566e-3	2	NC	5	NC	2
506			min	0	3	-.249	2	-.006	10	-6.598e-3	3	570.024	2	2573.814	1
507		7	max	.001	1	.116	3	.03	1	1.064e-2	2	NC	5	NC	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.211	2	-.009	10	-7.319e-3	3	685.816	2	3656.036	1
509	8	max	.001	1	.094	3	.026	3	1.171e-2	2	NC	5	NC	2
510		min	0	3	-.16	2	-.015	2	-8.04e-3	3	947.793	2	6757.46	3
511	9	max	.001	1	.072	3	.026	3	1.279e-2	2	NC	4	NC	1
512		min	0	3	-.113	2	-.025	2	-8.761e-3	3	1473.604	2	7047.264	3
513	10	max	.001	1	.063	3	.024	3	1.386e-2	2	NC	4	NC	4
514		min	0	3	-.091	2	-.029	2	-9.483e-3	3	1978.372	2	6184.787	2
515	11	max	.001	1	.072	3	.023	3	1.279e-2	2	NC	4	NC	1
516		min	0	3	-.113	2	-.025	2	-8.76e-3	3	1473.604	2	7770.614	2
517	12	max	.001	1	.094	3	.022	3	1.172e-2	2	NC	5	NC	2
518		min	0	3	-.16	2	-.015	2	-8.037e-3	3	947.793	2	8716.968	9
519	13	max	.001	1	.116	3	.03	1	1.064e-2	2	NC	5	NC	2
520		min	0	3	-.211	2	-.009	10	-7.315e-3	3	685.816	2	3650.115	1
521	14	max	.002	1	.132	3	.044	1	9.567e-3	2	NC	5	NC	2
522		min	0	3	-.249	2	-.006	10	-6.592e-3	3	570.024	2	2577.583	1
523	15	max	.002	1	.136	3	.049	1	8.493e-3	2	NC	5	NC	3
524		min	0	3	-.261	2	-.004	10	-5.87e-3	3	539.476	2	2356.263	1
525	16	max	.002	1	.126	3	.043	1	7.419e-3	2	NC	5	NC	3
526		min	0	3	-.243	2	-.003	10	-5.147e-3	3	584.975	2	2657.96	1
527	17	max	.002	1	.101	3	.028	1	6.346e-3	2	NC	5	NC	2
528		min	0	3	-.194	2	-.003	10	-4.424e-3	3	758.43	2	3853.733	1
529	18	max	.002	1	.064	3	.01	1	5.272e-3	2	NC	4	NC	2
530		min	0	3	-.118	2	-.004	10	-3.702e-3	3	1387.222	2	8823.856	1
531	19	max	.002	1	.02	3	.008	3	4.198e-3	2	NC	1	NC	1
532		min	0	3	-.028	2	-.008	2	-2.979e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.825e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.472e-5	2	NC	1	NC	1
535	2	max	0	3	-.001	15	0	1	8.366e-4	3	NC	1	NC	1
536		min	0	2	-.006	4	0	3	-5.154e-4	2	NC	1	NC	1
537	3	max	0	3	-.003	15	.003	1	1.291e-3	3	NC	3	NC	1
538		min	0	2	-.012	4	-.003	3	-9.76e-4	2	6143.049	4	NC	1
539	4	max	0	3	-.004	15	.006	1	1.745e-3	3	NC	5	NC	4
540		min	0	2	-.018	4	-.007	3	-1.437e-3	2	4214.49	4	6089.567	3
541	5	max	0	3	-.005	15	.01	1	2.199e-3	3	NC	15	NC	4
542		min	0	2	-.023	4	-.012	3	-1.897e-3	2	3288.61	4	4010.748	3
543	6	max	0	3	-.006	15	.015	1	2.653e-3	3	NC	15	NC	4
544		min	0	2	-.027	4	-.017	3	-2.358e-3	2	2767.713	4	2927.382	3
545	7	max	0	3	-.007	15	.019	1	3.107e-3	3	NC	15	NC	4
546		min	-.001	2	-.031	4	-.022	3	-2.819e-3	2	2454.462	4	2292.459	3
547	8	max	0	3	-.008	15	.024	1	3.561e-3	3	9641.864	15	NC	4
548		min	-.001	2	-.033	4	-.027	3	-3.279e-3	2	2266.464	4	1892.708	3
549	9	max	0	3	-.008	15	.027	1	4.016e-3	3	9211.381	15	NC	4
550		min	-.001	2	-.035	4	-.032	3	-3.74e-3	2	2165.273	4	1630.853	3
551	10	max	0	3	-.008	15	.031	1	4.47e-3	3	9075.197	15	NC	4
552		min	-.002	2	-.035	4	-.036	3	-4.201e-3	2	2133.261	4	1457.957	3
553	11	max	0	3	-.008	15	.033	1	4.924e-3	3	9211.381	15	NC	5
554		min	-.002	2	-.035	4	-.038	3	-4.661e-3	2	2165.273	4	1348.307	3
555	12	max	0	3	-.008	15	.033	1	5.378e-3	3	9641.864	15	NC	5
556		min	-.002	2	-.033	4	-.039	3	-5.122e-3	2	2266.464	4	1289.164	3
557	13	max	.001	3	-.007	15	.033	1	5.832e-3	3	NC	15	NC	5
558		min	-.002	2	-.031	4	-.038	3	-5.583e-3	2	2454.462	4	1276.895	3
559	14	max	.001	3	-.006	15	.03	1	6.286e-3	3	NC	15	NC	5
560		min	-.002	2	-.028	4	-.034	3	-6.043e-3	2	2767.713	4	1317.157	3
561	15	max	.001	3	-.005	15	.025	1	6.74e-3	3	NC	15	NC	4
562		min	-.003	2	-.023	4	-.028	3	-6.504e-3	2	3288.61	4	1430.486	3
563	16	max	.001	3	-.004	15	.018	1	7.195e-3	3	NC	5	NC	4
564		min	-.003	2	-.018	4	-.019	3	-6.965e-3	2	4214.49	4	1672.559	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	-.003	15	.007	1	7.649e-3	3	NC	3	NC	4
566		min	-.003	2	-.013	4	-.006	3	-7.425e-3	2	6143.049	4	2217.971	3
567	18	max	.001	3	0	2	.01	3	8.103e-3	3	NC	1	NC	4
568		min	-.003	2	-.007	4	-.013	2	-7.886e-3	2	NC	1	3949.863	3
569	19	max	.002	3	.005	2	.031	3	8.557e-3	3	NC	1	NC	1
570		min	-.003	2	-.002	9	-.031	2	-8.347e-3	2	NC	1	NC	1
571	M16A	1	max	0	0	10	.009	3	2.477e-3	3	NC	1	NC	1
572		min	-.001	3	-.001	9	-.009	2	-2.427e-3	2	NC	1	NC	1
573	2	max	0	10	-.002	15	.002	3	2.381e-3	3	NC	1	NC	1
574		min	-.001	3	-.007	4	-.003	2	-2.319e-3	2	NC	1	NC	1
575	3	max	0	10	-.003	15	.006	1	2.284e-3	3	NC	3	NC	4
576		min	-.001	3	-.013	4	-.003	3	-2.212e-3	2	6143.049	4	6352.551	3
577	4	max	0	10	-.004	15	.01	1	2.187e-3	3	NC	5	NC	4
578		min	-.001	3	-.018	4	-.008	3	-2.104e-3	2	4214.49	4	4838.182	3
579	5	max	0	10	-.005	15	.013	1	2.091e-3	3	NC	15	NC	4
580		min	-.001	3	-.023	4	-.01	3	-1.996e-3	2	3288.61	4	4185.036	3
581	6	max	0	10	-.006	15	.014	1	1.994e-3	3	NC	15	NC	4
582		min	-.001	3	-.027	4	-.012	3	-1.888e-3	2	2767.713	4	3904.019	3
583	7	max	0	10	-.007	15	.015	1	1.897e-3	3	NC	15	NC	4
584		min	0	3	-.031	4	-.013	3	-1.78e-3	2	2454.462	4	3842.524	3
585	8	max	0	10	-.008	15	.015	1	1.801e-3	3	9641.864	15	NC	4
586		min	0	3	-.033	4	-.013	3	-1.672e-3	2	2266.464	4	3949.423	3
587	9	max	0	10	-.008	15	.014	1	1.704e-3	3	9211.381	15	NC	4
588		min	0	3	-.035	4	-.012	3	-1.564e-3	2	2165.273	4	4219.921	3
589	10	max	0	10	-.008	15	.013	1	1.607e-3	3	9075.197	15	NC	4
590		min	0	3	-.035	4	-.011	3	-1.457e-3	2	2133.261	4	4683.578	3
591	11	max	0	10	-.008	15	.011	1	1.511e-3	3	9211.381	15	NC	4
592		min	0	3	-.035	4	-.01	3	-1.349e-3	2	2165.273	4	5411.817	3
593	12	max	0	10	-.008	15	.009	1	1.414e-3	3	9641.864	15	NC	4
594		min	0	3	-.033	4	-.008	3	-1.241e-3	2	2266.464	4	6547.293	3
595	13	max	0	10	-.007	15	.007	1	1.318e-3	3	NC	15	NC	2
596		min	0	3	-.03	4	-.006	3	-1.133e-3	2	2454.462	4	8333.846	1
597	14	max	0	10	-.006	15	.005	1	1.221e-3	3	NC	15	NC	1
598		min	0	3	-.027	4	-.004	3	-1.025e-3	2	2767.713	4	NC	1
599	15	max	0	10	-.005	15	.003	1	1.124e-3	3	NC	15	NC	1
600		min	0	3	-.023	4	-.002	3	-9.172e-4	2	3288.61	4	NC	1
601	16	max	0	10	-.004	15	.002	1	1.028e-3	3	NC	5	NC	1
602		min	0	3	-.018	4	0	3	-8.094e-4	2	4214.49	4	NC	1
603	17	max	0	10	-.003	15	0	14	9.309e-4	3	NC	3	NC	1
604		min	0	3	-.012	4	0	2	-7.015e-4	2	6143.049	4	NC	1
605	18	max	0	10	-.001	15	0	3	8.343e-4	3	NC	1	NC	1
606		min	0	3	-.006	4	0	2	-5.936e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	7.376e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.858e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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