

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

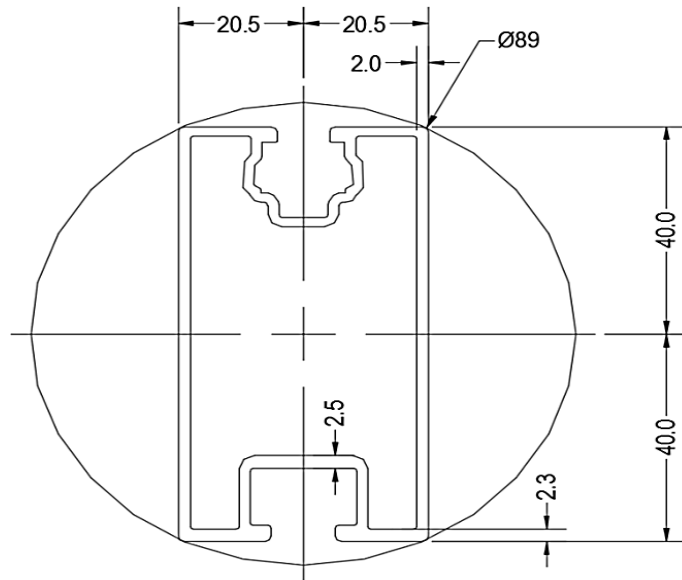
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

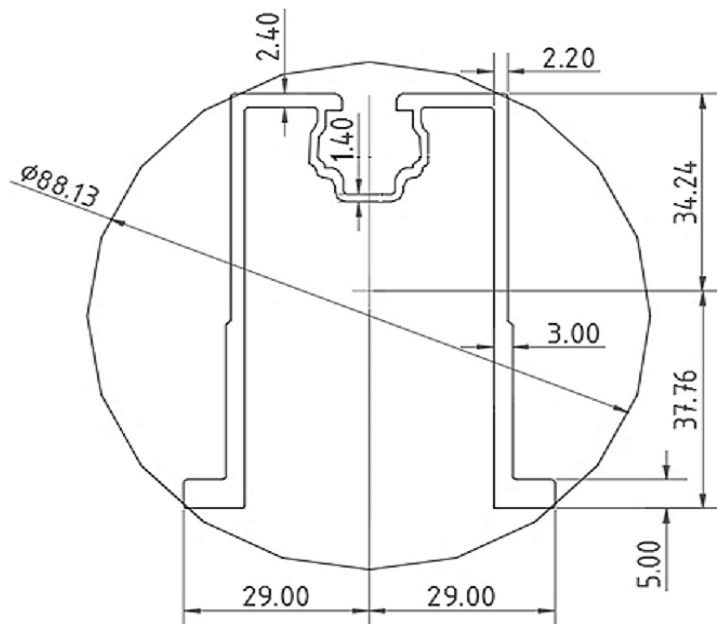
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	69 in
ΦF_{ty} STRONG-AXIS =	29.48 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.581 k-ft
M_z =	0.127 k-ft
$M_{y \text{ allowable}}$ =	1.831 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	47%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.45 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.521 k-ft
M_z =	0.000 k-ft
P_n =	0.292 k
$M_{y \text{ allowable}}$ =	1.445 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.037 k-ft
P_n =	0.209 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	10%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.475 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.882 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.004 k-ft
P_n =	0.211 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	11%



A cross brace kit is required every 18 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

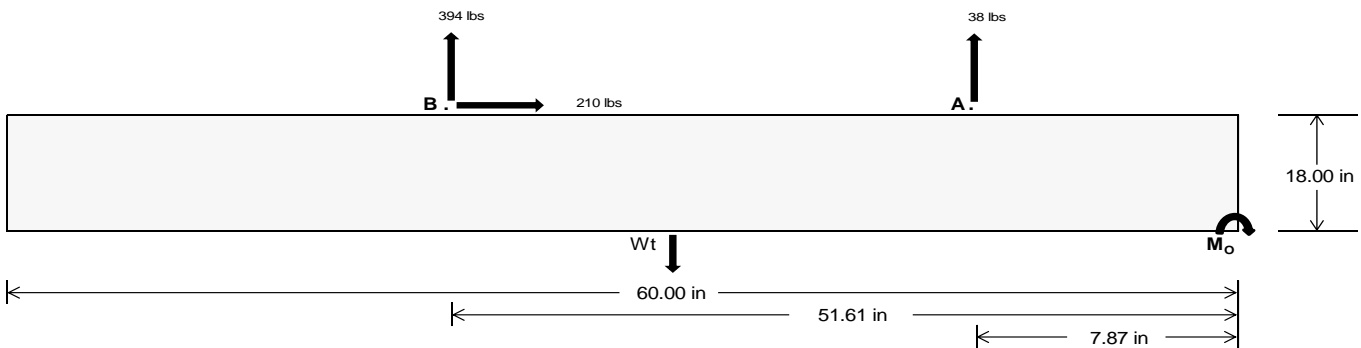
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	169.65	1711.20	k
Compressive Load =	1405.74	1225.47	k
Lateral Load =	30.61	911.42	k
Moment (Weak Axis) =	0.05	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24406.0$ in-lbs
Resisting Force Required = 813.53 lbs
S.F. = 1.67
Weight Required = 1355.89 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 210.22 lbs
Friction = 0.4
Weight Required = 525.55 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 210.22 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	504 lbs	504 lbs	504 lbs	504 lbs	453 lbs	453 lbs	453 lbs	453 lbs	676 lbs	676 lbs	676 lbs	676 lbs	-75 lbs	-75 lbs	-75 lbs	-75 lbs
F_B	357 lbs	357 lbs	357 lbs	357 lbs	494 lbs	494 lbs	494 lbs	494 lbs	608 lbs	608 lbs	608 lbs	608 lbs	-788 lbs	-788 lbs	-788 lbs	-788 lbs
F_V	47 lbs	47 lbs	47 lbs	47 lbs	378 lbs	378 lbs	378 lbs	378 lbs	315 lbs	315 lbs	315 lbs	315 lbs	-420 lbs	-420 lbs	-420 lbs	-420 lbs
P_{total}	2765 lbs	2855 lbs	2946 lbs	3037 lbs	2850 lbs	2940 lbs	3031 lbs	3121 lbs	3187 lbs	3278 lbs	3368 lbs	3459 lbs	279 lbs	334 lbs	388 lbs	442 lbs
M	356 lbs-ft	356 lbs-ft	356 lbs-ft	356 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	625 lbs-ft	625 lbs-ft	625 lbs-ft	625 lbs-ft	650 lbs-ft	650 lbs-ft	650 lbs-ft	650 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.33 ft	1.95 ft	1.67 ft	1.47 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	267.1 psf	264.8 psf	262.8 psf	260.9 psf	255.4 psf	253.7 psf	252.1 psf	250.7 psf	278.6 psf	275.8 psf	273.3 psf	270.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	364.9 psf	358.2 psf	352.0 psf	346.4 psf	395.9 psf	387.8 psf	380.4 psf	373.6 psf	449.9 psf	439.3 psf	429.7 psf	420.8 psf	614.3 psf	219.6 psf	163.5 psf	143.0 psf

Maximum Bearing Pressure = 614 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

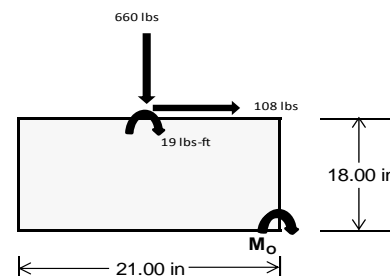
Overturning Check

$M_o = 396.6 \text{ ft-lbs}$
 Resisting Force Required = 453.20 lbs
 S.F. = 1.67
 Weight Required = 755.34 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	128 lbs	115 lbs	70 lbs	287 lbs	660 lbs	243 lbs	79 lbs	-11 lbs	24 lbs
F_v	17 lbs	143 lbs	17 lbs	11 lbs	108 lbs	13 lbs	17 lbs	143 lbs	17 lbs
P_{total}	2484 lbs	2471 lbs	2426 lbs	2530 lbs	2903 lbs	2486 lbs	768 lbs	678 lbs	713 lbs
M	48 lbs-ft	240 lbs-ft	51 lbs-ft	33 lbs-ft	181 lbs-ft	39 lbs-ft	49 lbs-ft	239 lbs-ft	49 lbs-ft
e	0.02 ft	0.10 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.06 ft	0.35 ft	0.07 ft
$L/6$	0.29 ft	1.56 ft	1.71 ft	1.72 ft	1.63 ft	1.72 ft	1.62 ft	1.04 ft	1.61 ft
f_{min}	265.0 sqft	188.5 sqft	257.5 sqft	276.2 sqft	260.8 sqft	268.9 sqft	68.7 sqft	-16.4 sqft	62.1 sqft
f_{max}	302.8 psf	376.3 psf	297.1 psf	302.2 psf	402.7 psf	299.3 psf	106.8 psf	171.2 psf	100.9 psf



Maximum Bearing Pressure = 403 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

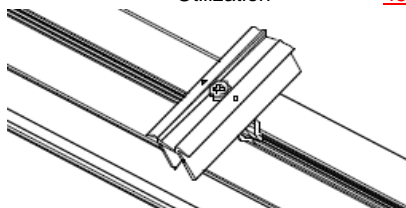
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

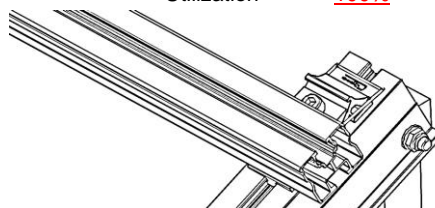
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.546 k
Allowable Uplift =	1.214 k
Utilization =	<u>45%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.113 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

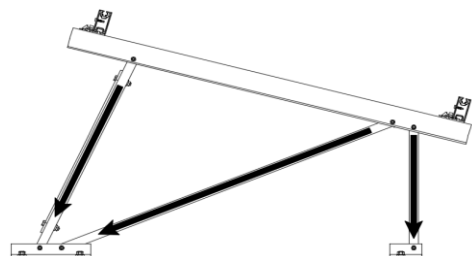
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.081 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Diagonal Strut

Maximum Axial Load =	0.475 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.170 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.211 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.078 in
	<u>0.078 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.427$$

$$143.909$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 69.00 \text{ in}$$

$$J = 0.427$$

$$156.378$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.831 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1669 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.445 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

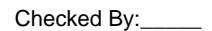
$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMMini 60 Cell 1V 25° 130mph 30psf 5.75ft 7-10Pa Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	277.373	1	.031	2	.36	1	0	10	0	4	0	15
30			min	-367.93	3	-.03	3	-.445	5	0	4	0	3	0	6
31		16	max	277.49	1	-.004	2	.36	1	0	10	0	1	0	15
32			min	-367.843	3	-.056	3	-.551	5	0	4	0	3	0	6
33		17	max	277.606	1	-.022	15	.36	1	0	10	0	1	0	15
34			min	-367.756	3	-.091	4	-.656	5	0	4	0	3	0	6
35		18	max	277.722	1	-.033	15	.36	1	0	10	0	1	0	15
36			min	-367.668	3	-.137	4	-.762	5	0	4	0	3	0	6
37		19	max	277.839	1	-.044	15	.36	1	0	10	0	1	0	15
38			min	-367.581	3	-.182	4	-.867	5	0	4	0	3	0	6
39	M3	1	max	125.58	2	1.775	6	-.027	10	0	5	.001	1	0	6
40			min	-130.137	3	.417	15	-1.398	4	0	1	0	10	0	15
41		2	max	125.511	2	1.598	6	-.027	10	0	5	.001	1	0	2
42			min	-130.189	3	.375	15	-1.264	4	0	1	0	10	0	15
43		3	max	125.443	2	1.421	6	-.027	10	0	5	0	1	0	2
44			min	-130.24	3	.333	15	-1.13	4	0	1	0	15	0	3
45		4	max	125.374	2	1.244	6	-.027	10	0	5	0	1	0	15
46			min	-130.292	3	.292	15	-.997	4	0	1	0	5	0	4
47		5	max	125.305	2	1.067	6	-.027	10	0	5	0	1	0	15
48			min	-130.343	3	.25	15	-.863	4	0	1	0	5	0	4
49		6	max	125.237	2	.889	6	-.027	10	0	5	0	1	0	15
50			min	-130.394	3	.208	15	-.73	4	0	1	0	5	0	4
51		7	max	125.168	2	.712	6	-.027	10	0	5	0	1	0	15
52			min	-130.446	3	.167	15	-.596	4	0	1	0	5	0	4
53		8	max	125.1	2	.535	6	-.027	10	0	5	0	1	0	15
54			min	-130.497	3	.125	15	-.462	4	0	1	0	5	-.001	4
55		9	max	125.031	2	.358	6	-.027	10	0	5	0	1	0	15
56			min	-130.549	3	.083	15	-.347	1	0	1	0	5	-.001	4
57		10	max	124.962	2	.181	6	-.027	10	0	5	0	1	0	15
58			min	-130.6	3	.042	15	-.347	1	0	1	0	5	-.001	4
59		11	max	124.894	2	.027	2	.013	5	0	5	0	1	0	15
60			min	-130.652	3	-.022	3	-.347	1	0	1	0	5	-.001	4
61		12	max	124.825	2	-.042	15	.147	5	0	5	0	1	0	15
62			min	-130.703	3	-.174	4	-.347	1	0	1	0	5	-.001	4
63		13	max	124.757	2	-.083	15	.28	5	0	5	0	1	0	15
64			min	-130.755	3	-.351	4	-.347	1	0	1	0	5	-.001	4
65		14	max	124.688	2	-.125	15	.414	5	0	5	0	1	0	15
66			min	-130.806	3	-.528	4	-.347	1	0	1	0	5	-.001	4
67		15	max	124.619	2	-.167	15	.548	5	0	5	0	1	0	15
68			min	-130.858	3	-.705	4	-.347	1	0	1	0	5	0	4
69		16	max	124.551	2	-.208	15	.681	5	0	5	0	12	0	15
70			min	-130.909	3	-.883	4	-.347	1	0	1	0	4	0	4
71		17	max	124.482	2	-.25	15	.815	5	0	5	0	12	0	15
72			min	-130.96	3	-1.06	4	-.347	1	0	1	0	4	0	4
73		18	max	124.414	2	-.292	15	.948	5	0	5	0	12	0	15
74			min	-131.012	3	-1.237	4	-.347	1	0	1	0	1	0	4
75		19	max	124.345	2	-.333	15	1.082	5	0	5	0	5	0	1
76			min	-131.063	3	-1.414	4	-.347	1	0	1	0	1	0	1
77	M4	1	max	388.13	1	0	1	-.102	10	0	1	0	5	0	1
78			min	-30.991	3	0	1	-22.486	4	0	1	0	2	0	1
79		2	max	388.194	1	0	1	-.102	10	0	1	0	12	0	1
80			min	-30.942	3	0	1	-22.543	4	0	1	-.002	4	0	1
81		3	max	388.259	1	0	1	-.102	10	0	1	0	12	0	1
82			min	-30.894	3	0	1	-22.599	4	0	1	-.004	4	0	1
83		4	max	388.324	1	0	1	-.102	10	0	1	0	10	0	1
84			min	-30.845	3	0	1	-22.655	4	0	1	-.006	4	0	1
85		5	max	388.388	1	0	1	-.102	10	0	1	0	10	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-30.797	3	0	1	-22.711	4	0	1	-.008	4	0	1
87		6	max	388.453	1	0	1	-.102	10	0	1	0	10	0	1
88			min	-30.748	3	0	1	-22.767	4	0	1	-.01	4	0	1
89		7	max	388.518	1	0	1	-.102	10	0	1	0	10	0	1
90			min	-30.7	3	0	1	-22.823	4	0	1	-.012	4	0	1
91		8	max	388.583	1	0	1	-.102	10	0	1	0	10	0	1
92			min	-30.651	3	0	1	-22.879	4	0	1	-.014	4	0	1
93		9	max	388.647	1	0	1	-.102	10	0	1	0	10	0	1
94			min	-30.603	3	0	1	-22.935	4	0	1	-.016	4	0	1
95		10	max	388.712	1	0	1	-.102	10	0	1	0	10	0	1
96			min	-30.554	3	0	1	-22.991	4	0	1	-.018	4	0	1
97		11	max	388.777	1	0	1	-.102	10	0	1	0	10	0	1
98			min	-30.506	3	0	1	-23.047	4	0	1	-.02	4	0	1
99		12	max	388.841	1	0	1	-.102	10	0	1	0	10	0	1
100			min	-30.457	3	0	1	-23.103	4	0	1	-.022	4	0	1
101		13	max	388.906	1	0	1	-.102	10	0	1	0	10	0	1
102			min	-30.409	3	0	1	-23.159	4	0	1	-.024	4	0	1
103		14	max	388.971	1	0	1	-.102	10	0	1	0	10	0	1
104			min	-30.36	3	0	1	-23.216	4	0	1	-.027	4	0	1
105		15	max	389.036	1	0	1	-.102	10	0	1	0	10	0	1
106			min	-30.312	3	0	1	-23.272	4	0	1	-.029	4	0	1
107		16	max	389.1	1	0	1	-.102	10	0	1	0	10	0	1
108			min	-30.263	3	0	1	-23.328	4	0	1	-.031	4	0	1
109		17	max	389.165	1	0	1	-.102	10	0	1	0	10	0	1
110			min	-30.215	3	0	1	-23.384	4	0	1	-.033	4	0	1
111		18	max	389.23	1	0	1	-.102	10	0	1	0	10	0	1
112			min	-30.166	3	0	1	-23.44	4	0	1	-.035	4	0	1
113		19	max	389.294	1	0	1	-.102	10	0	1	0	10	0	1
114			min	-30.117	3	0	1	-23.496	4	0	1	-.037	4	0	1
115	M6	1	max	880.356	1	.629	6	1.053	4	0	3	0	3	0	1
116			min	-1170.202	3	.142	15	-.182	3	0	5	0	9	0	1
117		2	max	880.472	1	.583	6	.947	4	0	3	0	4	0	15
118			min	-1170.114	3	.131	15	-.182	3	0	5	0	9	0	6
119		3	max	880.588	1	.537	6	.842	4	0	3	0	4	0	15
120			min	-1170.027	3	.121	15	-.182	3	0	5	0	10	0	6
121		4	max	880.705	1	.492	6	.736	4	0	3	0	4	0	15
122			min	-1169.94	3	.11	15	-.182	3	0	5	0	10	0	6
123		5	max	880.821	1	.449	2	.631	4	0	3	0	4	0	15
124			min	-1169.852	3	.099	15	-.182	3	0	5	0	10	0	6
125		6	max	880.938	1	.413	2	.525	4	0	3	0	4	0	15
126			min	-1169.765	3	.088	15	-.182	3	0	5	0	3	0	6
127		7	max	881.054	1	.377	2	.42	4	0	3	0	4	0	15
128			min	-1169.678	3	.078	15	-.182	3	0	5	0	3	0	6
129		8	max	881.17	1	.342	2	.314	4	0	3	0	4	0	15
130			min	-1169.591	3	.067	15	-.182	3	0	5	0	3	0	6
131		9	max	881.287	1	.306	2	.209	4	0	3	0	4	0	15
132			min	-1169.503	3	.05	12	-.182	3	0	5	0	3	0	2
133		10	max	881.403	1	.271	2	.12	1	0	3	0	4	0	15
134			min	-1169.416	3	.032	12	-.182	3	0	5	0	3	0	2
135		11	max	881.52	1	.235	2	.12	1	0	3	0	4	0	15
136			min	-1169.329	3	.012	3	-.182	3	0	5	0	3	0	2
137		12	max	881.636	1	.2	2	.12	1	0	3	0	4	0	15
138			min	-1169.241	3	-.014	3	-.182	3	0	5	0	3	0	2
139		13	max	881.752	1	.164	2	.12	1	0	3	0	4	0	15
140			min	-1169.154	3	-.041	3	-.247	5	0	5	0	3	0	2
141		14	max	881.869	1	.128	2	.12	1	0	3	0	4	0	15
142			min	-1169.067	3	-.068	3	-.353	5	0	5	0	3	0	2



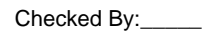
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	881.985	1	.093	2	.12	1	0	3	0	4	0	15
144		min	-1168.979	3	-.094	3	-.458	5	0	5	0	3	0	2
145	16	max	882.102	1	.057	2	.12	1	0	3	0	4	0	12
146		min	-1168.892	3	-.121	3	-.564	5	0	5	0	3	0	2
147	17	max	882.218	1	.022	2	.12	1	0	3	0	4	0	12
148		min	-1168.805	3	-.148	3	-.669	5	0	5	0	3	0	2
149	18	max	882.334	1	-.014	2	.12	1	0	3	0	4	0	12
150		min	-1168.718	3	-.175	3	-.775	5	0	5	0	3	0	2
151	19	max	882.451	1	-.05	2	.12	1	0	3	0	1	0	12
152		min	-1168.63	3	-.201	3	-.88	5	0	5	0	3	0	2
153	M7	1	max	474.621	2	1.789	.015	3	0	1	0	4	0	2
154		min	-390.419	3	.425	15	-1.383	4	0	3	0	3	0	12
155	2	max	474.553	2	1.612	4	.015	3	0	1	0	4	0	2
156		min	-390.471	3	.384	15	-1.249	4	0	3	0	3	0	3
157	3	max	474.484	2	1.435	4	.015	3	0	1	0	1	0	2
158		min	-390.522	3	.342	15	-1.116	4	0	3	0	3	0	3
159	4	max	474.416	2	1.258	4	.015	3	0	1	0	1	0	2
160		min	-390.574	3	.3	15	-.982	4	0	3	0	3	0	3
161	5	max	474.347	2	1.081	4	.015	3	0	1	0	1	0	15
162		min	-390.625	3	.259	15	-.848	4	0	3	0	5	0	3
163	6	max	474.278	2	.903	4	.015	3	0	1	0	1	0	15
164		min	-390.677	3	.217	15	-.715	4	0	3	0	5	0	6
165	7	max	474.21	2	.726	4	.015	3	0	1	0	1	0	15
166		min	-390.728	3	.175	15	-.581	4	0	3	0	5	0	6
167	8	max	474.141	2	.549	4	.015	3	0	1	0	1	0	15
168		min	-390.779	3	.134	15	-.448	4	0	3	0	5	0	6
169	9	max	474.073	2	.372	4	.015	3	0	1	0	1	0	15
170		min	-390.831	3	.09	12	-.314	4	0	3	0	5	-.001	6
171	10	max	474.004	2	.223	2	.015	3	0	1	0	1	0	15
172		min	-390.882	3	.021	12	-.18	4	0	3	0	5	-.001	6
173	11	max	473.935	2	.085	2	.015	3	0	1	0	1	0	15
174		min	-390.934	3	-.081	3	-.047	4	0	3	0	5	-.001	6
175	12	max	473.867	2	-.033	15	.087	5	0	1	0	1	0	15
176		min	-390.985	3	-.184	3	-.014	2	0	3	0	5	-.001	6
177	13	max	473.798	2	-.075	15	.221	5	0	1	0	1	0	15
178		min	-391.037	3	-.337	6	-.014	2	0	3	0	5	-.001	6
179	14	max	473.73	2	-.116	15	.354	5	0	1	0	1	0	15
180		min	-391.088	3	-.515	6	-.014	2	0	3	0	5	-.001	6
181	15	max	473.661	2	-.158	15	.488	5	0	1	0	1	0	15
182		min	-391.14	3	-.692	6	-.014	2	0	3	0	5	0	6
183	16	max	473.592	2	-.2	15	.621	5	0	1	0	1	0	15
184		min	-391.191	3	-.869	6	-.014	2	0	3	0	5	0	6
185	17	max	473.524	2	-.241	15	.755	5	0	1	0	1	0	15
186		min	-391.243	3	-1.046	6	-.014	2	0	3	0	5	0	6
187	18	max	473.455	2	-.283	15	.889	5	0	1	0	1	0	15
188		min	-391.294	3	-1.223	6	-.014	2	0	3	0	5	0	6
189	19	max	473.387	2	-.324	15	1.022	5	0	1	0	1	0	1
190		min	-391.345	3	-1.401	6	-.014	2	0	3	0	3	0	1
191	M8	1	max	1080.171	1	0	.595	1	0	1	0	4	0	1
192		min	-131.375	3	0	1	-22.764	4	0	1	0	1	0	1
193	2	max	1080.235	1	0	1	.595	1	0	1	0	1	0	1
194		min	-131.326	3	0	1	-22.82	4	0	1	-.002	4	0	1
195	3	max	1080.3	1	0	1	.595	1	0	1	0	1	0	1
196		min	-131.278	3	0	1	-22.876	4	0	1	-.004	4	0	1
197	4	max	1080.365	1	0	1	.595	1	0	1	0	1	0	1
198		min	-131.229	3	0	1	-22.932	4	0	1	-.006	4	0	1
199	5	max	1080.43	1	0	1	.595	1	0	1	0	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	279.929	1	.031	2	-.016	12	0	1	.001	5	0	15
258			min	-337.753	3	-.018	9	-.291	4	-.001	5	0	3	0	4
259		16	max	280.045	1	.011	5	-.016	12	0	1	0	5	0	15
260			min	-337.666	3	-.048	9	-.396	4	-.001	5	0	3	0	4
261		17	max	280.162	1	-.003	15	-.016	12	0	1	0	5	0	15
262			min	-337.579	3	-.077	9	-.502	4	-.001	5	0	1	0	4
263		18	max	280.278	1	-.014	15	-.016	12	0	1	0	5	0	15
264			min	-337.491	3	-.109	6	-.607	4	-.001	5	0	1	0	4
265		19	max	280.395	1	-.025	15	-.016	12	0	1	0	5	0	15
266			min	-337.404	3	-.155	6	-.713	4	-.001	5	0	1	0	4
267	M11	1	max	125.206	2	1.77	6	.386	1	.001	4	.001	5	0	6
268			min	-130.762	3	.412	15	-1.244	5	0	10	-.001	1	0	15
269		2	max	125.137	2	1.592	6	.386	1	.001	4	0	5	0	2
270			min	-130.813	3	.371	15	-1.11	5	0	10	-.001	1	0	12
271		3	max	125.068	2	1.415	6	.386	1	.001	4	0	5	0	2
272			min	-130.865	3	.329	15	-.977	5	0	10	0	1	0	3
273		4	max	125	2	1.238	6	.386	1	.001	4	0	5	0	15
274			min	-130.916	3	.287	15	-.843	5	0	10	0	1	0	4
275		5	max	124.931	2	1.061	6	.386	1	.001	4	0	3	0	15
276			min	-130.968	3	.246	15	-.71	5	0	10	0	1	0	4
277		6	max	124.863	2	.884	6	.386	1	.001	4	0	3	0	15
278			min	-131.019	3	.204	15	-.576	5	0	10	0	1	0	4
279		7	max	124.794	2	.706	6	.386	1	.001	4	0	3	0	15
280			min	-131.071	3	.162	15	-.442	5	0	10	0	1	0	4
281		8	max	124.725	2	.529	6	.386	1	.001	4	0	3	0	15
282			min	-131.122	3	.121	15	-.309	5	0	10	0	1	-.001	4
283		9	max	124.657	2	.352	6	.386	1	.001	4	0	3	0	15
284			min	-131.173	3	.079	15	-.175	5	0	10	0	1	-.001	4
285		10	max	124.588	2	.175	6	.386	1	.001	4	0	3	0	15
286			min	-131.225	3	.037	15	-.041	5	0	10	0	1	-.001	4
287		11	max	124.52	2	.027	2	.386	1	.001	4	0	3	0	15
288			min	-131.276	3	-.038	3	-.015	3	0	10	0	1	-.001	4
289		12	max	124.451	2	-.046	15	.386	1	.001	4	0	3	0	15
290			min	-131.328	3	-.18	4	-.015	3	0	10	0	4	-.001	4
291		13	max	124.382	2	-.087	15	.442	4	.001	4	0	3	0	15
292			min	-131.379	3	-.357	4	-.015	3	0	10	0	4	-.001	4
293		14	max	124.314	2	-.129	15	.576	4	.001	4	0	3	0	15
294			min	-131.431	3	-.534	4	-.015	3	0	10	0	1	-.001	4
295		15	max	124.245	2	-.171	15	.709	4	.001	4	0	3	0	15
296			min	-131.482	3	-.712	4	-.015	3	0	10	0	10	0	4
297		16	max	124.177	2	-.212	15	.843	4	.001	4	0	4	0	15
298			min	-131.534	3	-.889	4	-.015	3	0	10	0	10	0	4
299		17	max	124.108	2	-.254	15	.977	4	.001	4	0	4	0	15
300			min	-131.585	3	-1.066	4	-.015	3	0	10	0	10	0	4
301		18	max	124.039	2	-.296	15	1.11	4	.001	4	0	4	0	15
302			min	-131.637	3	-1.243	4	-.015	3	0	10	0	10	0	4
303		19	max	123.971	2	-.337	15	1.244	4	.001	4	0	4	0	1
304			min	-131.688	3	-1.42	4	-.015	3	0	10	0	10	0	1
305	M12	1	max	387.934	1	0	1	2.109	1	0	1	0	4	0	1
306			min	-30.513	3	0	1	-20.855	5	0	1	0	3	0	1
307		2	max	387.999	1	0	1	2.109	1	0	1	0	1	0	1
308			min	-30.464	3	0	1	-20.911	5	0	1	-.002	5	0	1
309		3	max	388.064	1	0	1	2.109	1	0	1	0	1	0	1
310			min	-30.416	3	0	1	-20.967	5	0	1	-.004	5	0	1
311		4	max	388.128	1	0	1	2.109	1	0	1	0	1	0	1
312			min	-30.367	3	0	1	-21.023	5	0	1	-.006	5	0	1
313		5	max	388.193	1	0	1	2.109	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-30.319	3	0	1	-21.079	5	0	1	-.007	5	0	1
315		6	max	388.258	1	0	1	2.109	1	0	1	0	1	0	1
316			min	-30.27	3	0	1	-21.135	5	0	1	-.009	5	0	1
317		7	max	388.323	1	0	1	2.109	1	0	1	.001	1	0	1
318			min	-30.222	3	0	1	-21.192	5	0	1	-.011	5	0	1
319		8	max	388.387	1	0	1	2.109	1	0	1	.001	1	0	1
320			min	-30.173	3	0	1	-21.248	5	0	1	-.013	5	0	1
321		9	max	388.452	1	0	1	2.109	1	0	1	.002	1	0	1
322			min	-30.125	3	0	1	-21.304	5	0	1	-.015	5	0	1
323		10	max	388.517	1	0	1	2.109	1	0	1	.002	1	0	1
324			min	-30.076	3	0	1	-21.36	5	0	1	-.017	5	0	1
325		11	max	388.581	1	0	1	2.109	1	0	1	.002	1	0	1
326			min	-30.028	3	0	1	-21.416	5	0	1	-.019	5	0	1
327		12	max	388.646	1	0	1	2.109	1	0	1	.002	1	0	1
328			min	-29.979	3	0	1	-21.472	5	0	1	-.021	5	0	1
329		13	max	388.711	1	0	1	2.109	1	0	1	.002	1	0	1
330			min	-29.931	3	0	1	-21.528	5	0	1	-.023	5	0	1
331		14	max	388.775	1	0	1	2.109	1	0	1	.002	1	0	1
332			min	-29.882	3	0	1	-21.584	5	0	1	-.025	5	0	1
333		15	max	388.84	1	0	1	2.109	1	0	1	.003	1	0	1
334			min	-29.834	3	0	1	-21.64	5	0	1	-.027	5	0	1
335		16	max	388.905	1	0	1	2.109	1	0	1	.003	1	0	1
336			min	-29.785	3	0	1	-21.696	5	0	1	-.029	5	0	1
337		17	max	388.97	1	0	1	2.109	1	0	1	.003	1	0	1
338			min	-29.737	3	0	1	-21.752	5	0	1	-.03	5	0	1
339		18	max	389.034	1	0	1	2.109	1	0	1	.003	1	0	1
340			min	-29.688	3	0	1	-21.808	5	0	1	-.032	5	0	1
341		19	max	389.099	1	0	1	2.109	1	0	1	.003	1	0	1
342			min	-29.639	3	0	1	-21.865	5	0	1	-.034	5	0	1
343	M1	1	max	106.525	1	346.524	3	-2.607	12	0	1	.081	1	.013	1
344			min	5.35	12	-277.133	1	-41.025	1	0	3	.006	10	-.014	3
345		2	max	106.643	1	346.334	3	-2.607	12	0	1	.072	1	.073	1
346			min	5.409	12	-277.386	1	-41.025	1	0	3	.005	10	-.089	3
347		3	max	68.83	1	5.699	14	-2.646	12	0	12	.062	1	.133	1
348			min	-5.565	10	-20.229	2	-40.856	1	0	1	.004	10	-.163	3
349		4	max	68.948	1	5.45	14	-2.646	12	0	12	.053	1	.133	1
350			min	-5.466	10	-20.482	2	-40.856	1	0	1	.004	10	-.158	3
351		5	max	69.066	1	5.211	9	-2.646	12	0	12	.044	1	.134	1
352			min	-5.368	10	-20.735	2	-40.856	1	0	1	.003	10	-.154	3
353		6	max	69.184	1	5	9	-2.646	12	0	12	.036	1	.136	2
354			min	-5.27	10	-20.988	2	-40.856	1	0	1	.003	10	-.15	3
355		7	max	69.302	1	4.789	9	-2.646	12	0	12	.027	1	.14	2
356			min	-5.171	10	-21.241	2	-40.856	1	0	1	.002	10	-.146	3
357		8	max	69.42	1	4.579	9	-2.646	12	0	12	.018	1	.145	2
358			min	-5.073	10	-21.494	2	-40.856	1	0	1	.001	10	-.141	3
359		9	max	69.538	1	4.368	9	-2.646	12	0	12	.009	1	.15	2
360			min	-4.975	10	-21.747	2	-40.856	1	0	1	0	10	-.137	3
361		10	max	69.656	1	4.157	9	-2.646	12	0	12	.001	4	.154	2
362			min	-4.876	10	-22	2	-40.856	1	0	1	0	10	-.132	3
363		11	max	69.774	1	3.946	9	-2.646	12	0	12	0	3	.159	2
364			min	-4.778	10	-22.254	2	-40.856	1	0	1	-.009	1	-.128	3
365		12	max	69.892	1	3.735	9	-2.646	12	0	12	0	12	.164	2
366			min	-4.68	10	-22.507	2	-40.856	1	0	1	-.018	1	-.123	3
367		13	max	70.01	1	3.524	9	-2.646	12	0	12	0	12	.169	2
368			min	-4.581	10	-22.76	2	-40.856	1	0	1	-.026	1	-.119	3
369		14	max	70.128	1	3.313	9	-2.646	12	0	12	-.002	12	.174	2
370			min	-4.483	10	-23.013	2	-40.856	1	0	1	-.035	1	-.114	3



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	70.246	1	3.102	9	-2.646	12	0	12	-.002	12	.179	2
372			min	-4.385	10	-23.266	2	-40.856	1	0	1	-.044	1	-.109	3
373		16	max	88.638	2	73.374	2	-2.675	12	0	1	-.003	12	.183	2
374			min	-19.831	3	-124.796	3	-41.159	1	0	5	-.053	1	-.104	3
375		17	max	88.756	2	73.121	2	-2.675	12	0	1	-.003	12	.167	2
376			min	-19.743	3	-124.986	3	-41.159	1	0	5	-.062	1	-.077	3
377		18	max	-4.331	12	350.422	2	-2.831	12	0	5	-.004	12	.092	2
378			min	-106.624	1	-153.304	3	-42.118	1	0	2	-.071	1	-.043	3
379		19	max	-4.272	12	350.168	2	-2.831	12	0	5	-.005	12	.016	2
380			min	-106.506	1	-153.494	3	-42.118	1	0	2	-.081	1	-.01	3
381	M5	1	max	245.261	1	1126.707	3	0	10	0	1	.039	4	.028	3
382			min	4.976	15	-899.198	1	-49.813	3	0	5	0	10	-.027	1
383		2	max	245.379	1	1126.517	3	0	10	0	1	.034	4	.169	1
384			min	5.012	15	-899.451	1	-49.813	3	0	5	-.004	3	-.216	3
385		3	max	175.699	3	6.459	9	5.482	3	0	3	.028	4	.36	1
386			min	-24.092	10	-68.609	2	-21.56	4	0	4	-.014	3	-.456	3
387		4	max	175.787	3	6.248	9	5.482	3	0	3	.024	4	.366	1
388			min	-23.994	10	-68.862	2	-21.318	4	0	4	-.013	3	-.442	3
389		5	max	175.876	3	6.037	9	5.482	3	0	3	.019	4	.372	1
390			min	-23.895	10	-69.115	2	-21.076	4	0	4	-.012	3	-.428	3
391		6	max	175.964	3	5.826	9	5.482	3	0	3	.015	4	.378	1
392			min	-23.797	10	-69.368	2	-20.834	4	0	4	-.011	3	-.415	3
393		7	max	176.053	3	5.616	9	5.482	3	0	3	.01	4	.386	2
394			min	-23.699	10	-69.621	2	-20.592	4	0	4	-.01	3	-.401	3
395		8	max	176.141	3	5.405	9	5.482	3	0	3	.006	4	.401	2
396			min	-23.6	10	-69.874	2	-20.35	4	0	4	-.008	3	-.387	3
397		9	max	176.23	3	5.194	9	5.482	3	0	3	.001	4	.417	2
398			min	-23.502	10	-70.127	2	-20.108	4	0	4	-.007	3	-.373	3
399		10	max	176.318	3	4.983	9	5.482	3	0	3	0	10	.432	2
400			min	-23.404	10	-70.38	2	-19.866	4	0	4	-.006	3	-.359	3
401		11	max	176.407	3	4.772	9	5.482	3	0	3	0	10	.447	2
402			min	-23.305	10	-70.633	2	-19.624	4	0	4	-.007	4	-.346	3
403		12	max	176.495	3	4.561	9	5.482	3	0	3	0	10	.462	2
404			min	-23.207	10	-70.887	2	-19.382	4	0	4	-.012	4	-.332	3
405		13	max	176.584	3	4.35	9	5.482	3	0	3	0	10	.478	2
406			min	-23.109	10	-71.14	2	-19.14	4	0	4	-.016	4	-.318	3
407		14	max	176.672	3	4.139	9	5.482	3	0	3	0	10	.493	2
408			min	-23.01	10	-71.393	2	-18.898	4	0	4	-.02	4	-.304	3
409		15	max	176.761	3	3.928	9	5.482	3	0	3	0	10	.509	2
410			min	-22.912	10	-71.646	2	-18.656	4	0	4	-.024	4	-.29	3
411		16	max	292.247	2	300.872	2	5.455	3	0	1	0	3	.521	2
412			min	-66.631	3	-371.163	3	-17.399	4	0	4	-.028	4	-.273	3
413		17	max	292.365	2	300.619	2	5.455	3	0	1	.002	3	.456	2
414			min	-66.542	3	-371.353	3	-17.157	4	0	4	-.032	4	-.192	3
415		18	max	-7.519	12	1133.641	2	5.024	3	0	4	.003	3	.213	2
416			min	-245.411	1	-491.084	3	-39.031	5	0	1	-.04	4	-.086	3
417		19	max	-7.46	12	1133.388	2	5.024	3	0	4	.004	3	.02	3
418			min	-245.292	1	-491.274	3	-38.789	5	0	1	-.049	4	-.033	2
419	M9	1	max	106.139	1	346.489	3	163.756	4	0	3	-.001	15	.013	1
420			min	1.978	15	-277.131	1	2.938	10	0	1	-.079	1	-.014	3
421		2	max	106.257	1	346.299	3	163.998	4	0	3	.032	5	.073	1
422			min	2.013	15	-277.384	1	2.938	10	0	1	-.07	1	-.089	3
423		3	max	68.941	1	5.606	9	39.824	1	0	1	.063	5	.132	1
424			min	-5.137	10	-20.244	2	-27.408	5	0	5	-.061	1	-.163	3
425		4	max	69.059	1	5.395	9	39.824	1	0	1	.057	5	.133	1
426			min	-5.039	10	-20.497	2	-27.166	5	0	5	-.052	1	-.158	3
427		5	max	69.177	1	5.184	9	39.824	1	0	1	.052	5	.134	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-4.94	10	-20.75	2	-26.924	5	0	5	-.043	1	-.154	3
429		6	max	69.295	1	4.973	9	39.824	1	0	1	.046	5	.136	2
430			min	-4.842	10	-21.003	2	-26.682	5	0	5	-.035	1	-.15	3
431		7	max	69.413	1	4.762	9	39.824	1	0	1	.04	5	.14	2
432			min	-4.744	10	-21.256	2	-26.44	5	0	5	-.026	1	-.146	3
433		8	max	69.531	1	4.551	9	39.824	1	0	1	.034	5	.145	2
434			min	-4.645	10	-21.509	2	-26.198	5	0	5	-.017	1	-.141	3
435		9	max	69.649	1	4.34	9	39.824	1	0	1	.029	5	.149	2
436			min	-4.547	10	-21.763	2	-25.956	5	0	5	-.009	1	-.137	3
437		10	max	69.767	1	4.129	9	39.824	1	0	1	.023	4	.154	2
438			min	-4.449	10	-22.016	2	-25.714	5	0	5	0	1	-.132	3
439		11	max	69.885	1	3.919	9	39.824	1	0	1	.019	4	.159	2
440			min	-4.35	10	-22.269	2	-25.472	5	0	5	0	10	-.128	3
441		12	max	70.003	1	3.708	9	39.824	1	0	1	.017	1	.164	2
442			min	-4.252	10	-22.522	2	-25.23	5	0	5	.001	10	-.123	3
443		13	max	70.121	1	3.497	9	39.824	1	0	1	.026	1	.169	2
444			min	-4.154	10	-22.775	2	-24.988	5	0	5	.002	10	-.119	3
445		14	max	70.239	1	3.286	9	39.824	1	0	1	.034	1	.174	2
446			min	-4.055	10	-23.028	2	-24.746	5	0	5	0	15	-.114	3
447		15	max	70.357	1	3.075	9	39.824	1	0	1	.043	1	.179	2
448			min	-3.957	10	-23.281	2	-24.504	5	0	5	-.004	5	-.109	3
449		16	max	88.843	2	73.067	2	40.169	1	0	10	.052	1	.183	2
450			min	-20.2	3	-125.226	3	-23.104	5	0	4	-.008	5	-.104	3
451		17	max	88.961	2	72.814	2	40.169	1	0	10	.061	1	.167	2
452			min	-20.111	3	-125.416	3	-22.862	5	0	4	-.013	5	-.077	3
453		18	max	4.855	5	350.422	2	42.277	1	0	2	.07	1	.092	2
454			min	-106.245	1	-153.3	3	-43.441	5	0	3	-.022	5	-.043	3
455		19	max	4.91	5	350.169	2	42.277	1	0	2	.079	1	.016	2
456			min	-106.127	1	-153.489	3	-43.199	5	0	3	-.032	5	-.01	3
457	M13	1	max	163.758	4	276.755	1	-1.977	15	.013	1	.079	1	0	1
458			min	2.939	10	-346.493	3	-106.132	1	-.014	3	.001	15	0	3
459		2	max	157.432	4	196.196	1	-1.047	15	.013	1	.02	1	.189	3
460			min	2.939	10	-245.406	3	-80.692	1	-.014	3	0	15	-.151	1
461		3	max	151.106	4	115.638	1	-.116	15	.013	1	.006	3	.314	3
462			min	2.939	10	-144.319	3	-55.251	1	-.014	3	-.024	1	-.251	1
463		4	max	144.78	4	35.079	1	1.148	5	.013	1	.003	3	.373	3
464			min	2.939	10	-43.233	3	-29.811	1	-.014	3	-.051	1	-.299	1
465		5	max	138.454	4	57.854	3	2.587	5	.013	1	0	5	.369	3
466			min	2.939	10	-45.48	1	-4.371	1	-.014	3	-.062	1	-.296	1
467		6	max	132.128	4	158.94	3	21.07	1	.013	1	.003	5	.3	3
468			min	2.939	10	-126.039	1	-1.055	3	-.014	3	-.056	1	-.241	1
469		7	max	125.802	4	260.027	3	46.51	1	.013	1	.006	5	.166	3
470			min	2.939	10	-206.598	1	.307	3	-.014	3	-.035	1	-.134	1
471		8	max	119.476	4	361.114	3	71.95	1	.013	1	.011	4	.023	1
472			min	2.939	10	-287.157	1	1.255	12	-.014	3	0	3	-.033	3
473		9	max	113.15	4	462.2	3	97.391	1	.013	1	.057	1	.232	1
474			min	2.939	10	-367.716	1	2.163	12	-.014	3	.001	12	-.296	3
475		10	max	106.824	4	563.287	3	122.831	1	.013	1	.127	1	.493	1
476			min	2.939	10	-448.274	1	3.071	12	-.014	3	.003	12	-.623	3
477		11	max	75.888	4	367.716	1	3.287	5	.014	3	.056	1	.232	1
478			min	2.607	12	-462.2	3	-97.005	1	-.013	1	-.016	5	-.296	3
479		12	max	69.562	4	287.157	1	4.727	5	.014	3	.003	2	.023	1
480			min	2.607	12	-361.114	3	-71.564	1	-.013	1	-.014	5	-.033	3
481		13	max	63.235	4	206.598	1	6.166	5	.014	3	-.003	10	.166	3
482			min	2.607	12	-260.027	3	-46.124	1	-.013	1	-.035	1	-.134	1
483		14	max	56.909	4	126.039	1	7.606	5	.014	3	-.004	15	.3	3
484			min	2.607	12	-158.94	3	-20.684	1	-.013	1	-.057	1	-.241	1



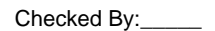
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	50.583	4	45.48	1	10.297	4	.014	3	0	15	.369	3
486			min	2.607	12	-57.854	3	-.406	10	-.013	1	-.062	1	-.296	1
487		16	max	44.257	4	43.233	3	30.197	1	.014	3	.006	5	.373	3
488			min	2.607	12	-35.079	1	2.164	10	-.013	1	-.05	1	-.299	1
489		17	max	41.128	1	144.319	3	55.637	1	.014	3	.013	5	.314	3
490			min	2.607	12	-115.638	1	3.535	12	-.013	1	-.023	1	-.251	1
491		18	max	41.128	1	245.406	3	81.078	1	.014	3	.026	4	.189	3
492			min	2.607	12	-196.196	1	4.443	12	-.013	1	0	10	-.151	1
493		19	max	41.128	1	346.493	3	106.518	1	.014	3	.081	1	0	1
494			min	2.607	12	-276.755	1	5.351	12	-.013	1	.006	10	0	3
495	M16	1	max	43.189	5	350.347	2	4.91	5	.01	3	.079	1	0	2
496			min	-42.169	1	-153.512	3	-106.135	1	-.016	2	-.032	5	0	3
497		2	max	36.863	5	248.38	2	6.35	5	.01	3	.02	1	.084	3
498			min	-42.169	1	-109.157	3	-80.694	1	-.016	2	-.028	5	-.191	2
499		3	max	30.537	5	146.414	2	7.789	5	.01	3	0	12	.139	3
500			min	-42.169	1	-64.802	3	-55.254	1	-.016	2	-.029	4	-.317	2
501		4	max	24.211	5	44.447	2	9.229	5	.01	3	-.002	12	.167	3
502			min	-42.169	1	-20.447	3	-29.814	1	-.016	2	-.051	1	-.378	2
503		5	max	17.885	5	23.908	3	10.668	5	.01	3	-.003	12	.166	3
504			min	-42.169	1	-57.52	2	-4.373	1	-.016	2	-.062	1	-.374	2
505		6	max	11.559	5	68.264	3	21.067	1	.01	3	-.003	15	.136	3
506			min	-42.169	1	-159.486	2	-.241	3	-.016	2	-.056	1	-.305	2
507		7	max	5.233	5	112.619	3	46.507	1	.01	3	.004	5	.078	3
508			min	-42.169	1	-261.453	2	.836	12	-.016	2	-.035	1	-.17	2
509		8	max	.569	3	156.974	3	71.948	1	.01	3	.013	4	.029	2
510			min	-42.169	1	-363.42	2	1.744	12	-.016	2	-.004	3	-.008	3
511		9	max	.569	3	201.329	3	97.388	1	.01	3	.057	1	.294	2
512			min	-42.169	1	-465.386	2	2.652	12	-.016	2	-.002	3	-.122	3
513		10	max	25.058	5	-11.097	15	122.829	1	.004	14	.127	1	.624	2
514			min	-42.169	1	-567.353	2	-5.778	3	-.016	2	.003	12	-.265	3
515		11	max	18.732	5	465.386	2	2.811	5	.016	2	.056	1	.294	2
516			min	-42.016	1	-201.329	3	-97.009	1	-.01	3	-.013	5	-.122	3
517		12	max	12.406	5	363.42	2	4.251	5	.016	2	.003	2	.029	2
518			min	-42.016	1	-156.974	3	-71.568	1	-.01	3	-.011	5	-.008	3
519		13	max	6.08	5	261.453	2	5.69	5	.016	2	-.001	12	.078	3
520			min	-42.016	1	-112.619	3	-46.128	1	-.01	3	-.035	1	-.17	2
521		14	max	-.103	15	159.486	2	7.13	5	.016	2	-.002	12	.136	3
522			min	-42.016	1	-68.264	3	-20.688	1	-.01	3	-.056	1	-.305	2
523		15	max	-2.831	12	57.52	2	9.796	4	.016	2	.001	5	.166	3
524			min	-42.016	1	-23.908	3	-.399	10	-.01	3	-.062	1	-.374	2
525		16	max	-2.831	12	20.447	3	30.193	1	.016	2	.007	5	.167	3
526			min	-42.016	1	-44.447	2	1.548	12	-.01	3	-.05	1	-.378	2
527		17	max	-2.831	12	64.802	3	55.634	1	.016	2	.014	5	.139	3
528			min	-42.016	1	-146.414	2	2.456	12	-.01	3	-.023	1	-.317	2
529		18	max	-2.831	12	109.157	3	81.074	1	.016	2	.027	4	.084	3
530			min	-42.016	1	-248.381	2	3.364	12	-.01	3	0	10	-.191	2
531		19	max	-2.831	12	153.512	3	106.514	1	.016	2	.081	1	0	2
532			min	-42.016	1	-350.347	2	4.272	12	-.01	3	.005	12	0	5
533	M15	1	max	0	1	1.309	9	.062	3	0	9	0	9	0	1
534			min	-62.246	3	0	1	-.022	9	0	3	0	3	0	1
535		2	max	0	1	1.164	9	.062	3	0	9	0	9	0	1
536			min	-62.311	3	0	1	-.022	9	0	3	0	3	0	9
537		3	max	0	1	1.018	9	.062	3	0	9	0	9	0	1
538			min	-62.376	3	0	1	-.022	9	0	3	0	3	0	9
539		4	max	0	1	.873	9	.062	3	0	9	0	9	0	1
540			min	-62.441	3	0	1	-.022	9	0	3	0	3	-.001	9
541		5	max	0	1	.727	9	.062	3	0	9	0	9	0	1



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 25° 130mph 30psf 5.75ft 7-10Pa Page 31



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.098	2	0	10	.031	1	0	3	0	4	0	10
600		min	-210.438	4	-1.492	4	-.101	5	0	2	0	3	-.003	4
601	16	max	.185	2	0	10	.031	1	0	3	0	4	0	10
602		min	-210.475	4	-1.791	4	-.127	5	0	2	0	3	-.002	4
603	17	max	.272	2	0	10	.031	1	0	3	0	1	0	10
604		min	-210.513	4	-2.089	4	-.152	5	0	2	0	3	-.002	4
605	18	max	.359	2	0	10	.031	1	0	3	0	1	0	10
606		min	-210.551	4	-2.387	4	-.178	5	0	2	0	5	0	4
607	19	max	.446	2	0	10	.031	1	0	3	0	1	0	1
608		min	-210.589	4	-2.686	4	-.204	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.009	2	.008	1	1.276e-3	5	NC	3	NC	2	
2			min	-.003	3	-.008	3	-.013	5	-6.235e-4	1	4105.946	2	4392.654	1	
3			2	max	.002	1	.008	2	.008	1	1.299e-3	5	NC	3	NC	2
4				min	-.003	3	-.008	3	-.012	5	-5.976e-4	1	4460.241	2	4746.502	1
5			3	max	.002	1	.007	2	.007	1	1.322e-3	5	NC	3	NC	2
6				min	-.003	3	-.008	3	-.012	5	-5.717e-4	1	4877.905	2	5163.797	1
7			4	max	.002	1	.007	2	.006	1	1.346e-3	5	NC	3	NC	2
8				min	-.003	3	-.007	3	-.012	5	-5.457e-4	1	5373.53	2	5659.893	1
9			5	max	.002	1	.006	2	.006	1	1.369e-3	5	NC	1	NC	2
10				min	-.003	3	-.007	3	-.011	5	-5.198e-4	1	5966.366	2	6255.297	1
11			6	max	.002	1	.005	2	.005	1	1.392e-3	5	NC	1	NC	2
12				min	-.002	3	-.007	3	-.011	5	-4.939e-4	1	6682.212	2	6977.894	1
13			7	max	.002	1	.005	2	.005	1	1.415e-3	5	NC	1	NC	2
14				min	-.002	3	-.006	3	-.01	5	-4.68e-4	1	7556.29	2	7866.404	1
15			8	max	.002	1	.004	2	.004	1	1.438e-3	5	NC	1	NC	2
16				min	-.002	3	-.006	3	-.009	5	-4.42e-4	1	8637.739	2	8975.889	1
17			9	max	.001	1	.004	2	.004	1	1.461e-3	5	NC	1	NC	1
18				min	-.002	3	-.005	3	-.009	5	-4.161e-4	1	9996.874	2	NC	1
19			10	max	.001	1	.003	2	.003	1	1.484e-3	5	NC	1	NC	1
20			min	-.002	3	-.005	3	-.008	5	-3.902e-4	1	NC	1	NC	1	
21		11	max	.001	1	.003	2	.002	1	1.507e-3	5	NC	1	NC	1	
22			min	-.001	3	-.004	3	-.007	5	-3.643e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	1.531e-3	5	NC	1	NC	1	
24			min	-.001	3	-.004	3	-.007	5	-3.383e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.002	1	1.554e-3	5	NC	1	NC	1	
26			min	-.001	3	-.003	3	-.006	5	-3.124e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.001	1	1.577e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.005	5	-2.865e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.6e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.004	5	-2.606e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.623e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.003	5	-2.346e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.646e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-2.087e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.669e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-1.828e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.692e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.569e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	7.296e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-7.874e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.004	5	9.026e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-7.927e-4	5	NC	1	NC	1



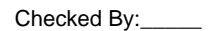
Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.008	5	1.076e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-7.98e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.012	5	1.248e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-8.034e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.016	5	1.421e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-8.087e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.02	4	1.594e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-8.14e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.025	4	1.767e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-8.194e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.029	4	1.94e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-8.247e-4	5	NC	1	NC	1
55		9	max	0	3	.002	2	.033	4	2.113e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-8.3e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.036	4	2.286e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-8.354e-4	5	NC	1	NC	1
59		11	max	0	3	.003	2	.04	4	2.459e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-8.407e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.044	4	2.632e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-8.46e-4	5	NC	1	NC	1
63		13	max	0	3	.004	2	.048	4	2.805e-4	1	NC	1	NC	1
64			min	0	2	-.008	3	0	10	-8.513e-4	5	NC	1	NC	1
65		14	max	.001	3	.005	2	.051	4	2.978e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	10	-8.567e-4	5	9624.879	2	NC	1
67		15	max	.001	3	.006	2	.055	4	3.151e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	-8.62e-4	5	8123.978	2	NC	1
69		16	max	.001	3	.007	2	.058	4	3.324e-4	1	NC	1	NC	1
70			min	-.001	2	-.008	3	0	10	-8.673e-4	5	6954.635	2	NC	1
71		17	max	.001	3	.008	2	.062	4	3.497e-4	1	NC	3	NC	1
72			min	-.001	2	-.008	3	0	10	-8.727e-4	5	6034.719	2	NC	1
73		18	max	.001	3	.009	2	.065	4	3.669e-4	1	NC	3	NC	1
74			min	-.001	2	-.008	3	0	10	-8.78e-4	5	5304.913	2	NC	1
75		19	max	.001	3	.01	2	.068	4	3.842e-4	1	NC	3	NC	1
76			min	-.001	2	-.008	3	0	10	-8.833e-4	5	4722.106	2	NC	1
77	M4	1	max	.002	1	.01	2	0	10	4.065e-3	5	NC	1	NC	2
78			min	0	3	-.008	3	-.072	4	-5.21e-4	1	NC	1	267.862	4
79		2	max	.002	1	.01	2	0	10	4.065e-3	5	NC	1	NC	2
80			min	0	3	-.008	3	-.066	4	-5.21e-4	1	NC	1	291.993	4
81		3	max	.002	1	.009	2	0	10	4.065e-3	5	NC	1	NC	2
82			min	0	3	-.007	3	-.06	4	-5.21e-4	1	NC	1	320.714	4
83		4	max	.002	1	.008	2	0	10	4.065e-3	5	NC	1	NC	2
84			min	0	3	-.007	3	-.054	4	-5.21e-4	1	NC	1	355.233	4
85		5	max	.001	1	.008	2	0	10	4.065e-3	5	NC	1	NC	1
86			min	0	3	-.006	3	-.049	4	-5.21e-4	1	NC	1	397.197	4
87		6	max	.001	1	.007	2	0	10	4.065e-3	5	NC	1	NC	1
88			min	0	3	-.006	3	-.043	4	-5.21e-4	1	NC	1	448.894	4
89		7	max	.001	1	.007	2	0	10	4.065e-3	5	NC	1	NC	1
90			min	0	3	-.006	3	-.038	4	-5.21e-4	1	NC	1	513.588	4
91		8	max	.001	1	.006	2	0	10	4.065e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.032	4	-5.21e-4	1	NC	1	596.051	4
93		9	max	.001	1	.006	2	0	10	4.065e-3	5	NC	1	NC	1
94			min	0	3	-.005	3	-.027	4	-5.21e-4	1	NC	1	703.507	4
95		10	max	0	1	.005	2	0	10	4.065e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.023	4	-5.21e-4	1	NC	1	847.309	4
97		11	max	0	1	.004	2	0	10	4.065e-3	5	NC	1	NC	1
98			min	0	3	-.004	3	-.018	4	-5.21e-4	1	NC	1	1046.154	4
99		12	max	0	1	.004	2	0	10	4.065e-3	5	NC	1	NC	1





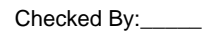
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.009	5	2.298e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-8.06e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.013	5	1.805e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-7.971e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.017	5	1.311e-5	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-7.882e-4	4	9234.078	2	NC	1
163		6	max	.001	3	.006	2	.021	5	1.564e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	1	-7.793e-4	4	7407.832	2	NC	1
165		7	max	.001	3	.007	2	.026	5	3.274e-5	3	NC	3	NC	1
166			min	-.002	2	-.01	3	0	1	-7.704e-4	4	6156.463	2	NC	1
167		8	max	.002	3	.009	2	.03	4	4.983e-5	3	NC	3	NC	1
168			min	-.002	2	-.011	3	-.001	1	-7.615e-4	4	5237.179	2	NC	1
169		9	max	.002	3	.01	2	.034	4	6.693e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	-.001	1	-7.527e-4	4	4529.106	2	NC	1
171		10	max	.002	3	.012	2	.038	4	8.402e-5	3	NC	3	NC	1
172			min	-.003	2	-.014	3	-.001	1	-7.438e-4	4	3965.295	2	NC	1
173		11	max	.002	3	.013	2	.042	4	1.011e-4	3	NC	3	NC	1
174			min	-.003	2	-.015	3	-.001	1	-7.349e-4	4	3505.554	2	NC	1
175		12	max	.003	3	.015	2	.045	4	1.182e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-7.26e-4	4	3124.149	2	NC	1
177		13	max	.003	3	.016	2	.049	4	1.353e-4	3	NC	3	NC	1
178			min	-.004	2	-.018	3	-.002	1	-7.171e-4	4	2803.695	2	NC	1
179		14	max	.003	3	.018	2	.053	4	1.524e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	-.002	1	-7.082e-4	4	2531.907	2	NC	1
181		15	max	.003	3	.02	2	.056	4	1.695e-4	3	NC	3	NC	1
182			min	-.004	2	-.019	3	-.002	1	-6.993e-4	4	2299.77	2	NC	1
183		16	max	.004	3	.022	2	.06	4	1.866e-4	3	NC	3	NC	1
184			min	-.004	2	-.02	3	-.002	1	-6.905e-4	4	2100.462	2	NC	1
185		17	max	.004	3	.024	2	.063	4	2.037e-4	3	NC	3	NC	1
186			min	-.005	2	-.021	3	-.002	1	-6.816e-4	4	1928.694	2	NC	1
187		18	max	.004	3	.026	2	.066	4	2.208e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	-.002	1	-6.727e-4	4	1780.282	2	NC	1
189		19	max	.004	3	.028	2	.07	4	2.379e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	-.002	1	-6.638e-4	4	1651.873	2	NC	1
191	M8	1	max	.005	1	.03	2	.002	1	3.86e-3	4	NC	1	NC	1
192			min	0	3	-.023	3	-.073	4	-1.81e-4	3	NC	1	264.666	4
193		2	max	.005	1	.028	2	.002	1	3.86e-3	4	NC	1	NC	1
194			min	0	3	-.022	3	-.067	4	-1.81e-4	3	NC	1	288.509	4
195		3	max	.005	1	.027	2	.002	1	3.86e-3	4	NC	1	NC	1
196			min	0	3	-.02	3	-.061	4	-1.81e-4	3	NC	1	316.888	4
197		4	max	.004	1	.025	2	.001	1	3.86e-3	4	NC	1	NC	1
198			min	0	3	-.019	3	-.055	4	-1.81e-4	3	NC	1	350.995	4
199		5	max	.004	1	.023	2	.001	1	3.86e-3	4	NC	1	NC	1
200			min	0	3	-.018	3	-.049	4	-1.81e-4	3	NC	1	392.459	4
201		6	max	.004	1	.022	2	.001	1	3.86e-3	4	NC	1	NC	1
202			min	0	3	-.016	3	-.044	4	-1.81e-4	3	NC	1	443.541	4
203		7	max	.003	1	.02	2	0	1	3.86e-3	4	NC	1	NC	1
204			min	0	3	-.015	3	-.038	4	-1.81e-4	3	NC	1	507.463	4
205		8	max	.003	1	.018	2	0	1	3.86e-3	4	NC	1	NC	1
206			min	0	3	-.014	3	-.033	4	-1.81e-4	3	NC	1	588.945	4
207		9	max	.003	1	.017	2	0	1	3.86e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.028	4	-1.81e-4	3	NC	1	695.122	4
209		10	max	.003	1	.015	2	0	1	3.86e-3	4	NC	1	NC	1
210			min	0	3	-.011	3	-.023	4	-1.81e-4	3	NC	1	837.211	4
211		11	max	.002	1	.013	2	0	1	3.86e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.019	4	-1.81e-4	3	NC	1	1033.689	4
213		12	max	.002	1	.012	2	0	1	3.86e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.007	4	3.593e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-7.66e-4	4	NC	1	NC	1
273	4	max	0	3	0	2	.01	4	1.828e-5	3	NC	1	NC	1
274		min	0	2	-.003	3	0	3	-8.322e-4	4	NC	1	NC	1
275	5	max	0	3	0	2	.013	4	6.249e-7	3	NC	1	NC	1
276		min	0	2	-.003	3	-.001	3	-8.984e-4	4	NC	1	NC	1
277	6	max	0	3	0	2	.017	5	-1.09e-5	12	NC	1	NC	1
278		min	0	2	-.004	3	-.001	3	-9.646e-4	4	NC	1	NC	1
279	7	max	0	3	0	2	.02	5	-1.32e-5	10	NC	1	NC	1
280		min	0	2	-.005	3	-.002	3	-1.031e-3	4	NC	1	NC	1
281	8	max	0	3	.001	2	.024	5	-1.482e-5	10	NC	1	NC	1
282		min	0	2	-.006	3	-.002	3	-1.097e-3	4	NC	1	NC	1
283	9	max	0	3	.002	2	.027	5	-1.644e-5	10	NC	1	NC	1
284		min	0	2	-.006	3	-.002	3	-1.163e-3	4	NC	1	NC	1
285	10	max	0	3	.002	2	.031	5	-1.806e-5	10	NC	1	NC	1
286		min	0	2	-.007	3	-.002	1	-1.229e-3	4	NC	1	NC	1
287	11	max	0	3	.003	2	.034	5	-1.968e-5	10	NC	1	NC	1
288		min	0	2	-.007	3	-.003	1	-1.295e-3	4	NC	1	NC	1
289	12	max	0	3	.003	2	.037	5	-2.13e-5	10	NC	1	NC	1
290		min	0	2	-.008	3	-.004	1	-1.362e-3	4	NC	1	NC	1
291	13	max	0	3	.004	2	.041	5	-2.292e-5	10	NC	1	NC	1
292		min	0	2	-.008	3	-.004	1	-1.428e-3	4	NC	1	NC	1
293	14	max	.001	3	.005	2	.044	5	-2.453e-5	10	NC	1	NC	2
294		min	-.001	2	-.008	3	-.005	1	-1.494e-3	4	9640.454	2	9385.562	1
295	15	max	.001	3	.006	2	.047	5	-2.615e-5	10	NC	1	NC	2
296		min	-.001	2	-.008	3	-.006	1	-1.56e-3	4	8135.856	2	8319.457	1
297	16	max	.001	3	.007	2	.051	5	-2.777e-5	10	NC	1	NC	2
298		min	-.001	2	-.008	3	-.006	1	-1.626e-3	4	6963.91	2	7478.029	1
299	17	max	.001	3	.008	2	.054	5	-2.939e-5	10	NC	3	NC	2
300		min	-.001	2	-.008	3	-.007	1	-1.693e-3	4	6042.13	2	6805.51	1
301	18	max	.001	3	.009	2	.058	5	-3.101e-5	10	NC	3	NC	2
302		min	-.001	2	-.008	3	-.007	1	-1.759e-3	4	5310.969	2	6263.21	1
303	19	max	.001	3	.01	2	.061	5	-3.263e-5	10	NC	3	NC	2
304		min	-.001	2	-.008	3	-.008	1	-1.825e-3	4	4727.165	2	5823.713	1
305	M12	1	max	.002	1	.01	.007	1	4.873e-3	4	NC	1	NC	3
306		min	0	3	-.008	3	-.067	5	3.5e-5	10	NC	1	288.511	5
307	2	max	.002	1	.01	2	.006	1	4.873e-3	4	NC	1	NC	2
308		min	0	3	-.008	3	-.061	5	3.5e-5	10	NC	1	314.497	5
309	3	max	.002	1	.009	2	.006	1	4.873e-3	4	NC	1	NC	2
310		min	0	3	-.007	3	-.056	5	3.5e-5	10	NC	1	345.425	5
311	4	max	.002	1	.008	2	.005	1	4.873e-3	4	NC	1	NC	2
312		min	0	3	-.007	3	-.051	5	3.5e-5	10	NC	1	382.595	5
313	5	max	.001	1	.008	2	.005	1	4.873e-3	4	NC	1	NC	2
314		min	0	3	-.007	3	-.045	5	3.5e-5	10	NC	1	427.78	5
315	6	max	.001	1	.007	2	.004	1	4.873e-3	4	NC	1	NC	2
316		min	0	3	-.006	3	-.04	5	3.5e-5	10	NC	1	483.447	5
317	7	max	.001	1	.007	2	.003	1	4.873e-3	4	NC	1	NC	2
318		min	0	3	-.006	3	-.035	5	3.5e-5	10	NC	1	553.105	5
319	8	max	.001	1	.006	2	.003	1	4.873e-3	4	NC	1	NC	2
320		min	0	3	-.005	3	-.03	5	3.5e-5	10	NC	1	641.897	5
321	9	max	.001	1	.006	2	.003	1	4.873e-3	4	NC	1	NC	2
322		min	0	3	-.005	3	-.026	5	3.5e-5	10	NC	1	757.597	5
323	10	max	0	1	.005	2	.002	1	4.873e-3	4	NC	1	NC	2
324		min	0	3	-.004	3	-.021	5	3.5e-5	10	NC	1	912.429	5
325	11	max	0	1	.004	2	.002	1	4.873e-3	4	NC	1	NC	1
326		min	0	3	-.004	3	-.017	5	3.5e-5	10	NC	1	1126.523	5
327	12	max	0	1	.004	2	.001	1	4.873e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.003	3	-.013	5	3.5e-5	10	NC	1	1434.991	5
329		13	max	0	1	.003	2	.001	1	4.873e-3	4	NC	1	NC	1
330			min	0	3	-.003	3	-.01	5	3.5e-5	10	NC	1	1903.693	5
331		14	max	0	1	.003	2	0	1	4.873e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.007	5	3.5e-5	10	NC	1	2668.475	5
333		15	max	0	1	.002	2	0	1	4.873e-3	4	NC	1	NC	1
334			min	0	3	-.002	3	-.005	5	3.5e-5	10	NC	1	4048.478	5
335		16	max	0	1	.002	2	0	1	4.873e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.003	5	3.5e-5	10	NC	1	6950.917	5
337		17	max	0	1	.001	2	0	1	4.873e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.001	5	3.5e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.873e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	3.5e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.873e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	3.5e-5	10	NC	1	NC	1
343	M1	1	max	.008	3	.025	3	.007	5	6.889e-3	1	NC	1	NC	1
344			min	-.008	2	-.023	2	-.003	1	-8.488e-3	3	NC	1	NC	1
345		2	max	.008	3	.014	3	.01	5	3.217e-3	1	NC	4	NC	1
346			min	-.008	2	-.013	2	-.006	1	-4.188e-3	3	4372.756	2	NC	1
347		3	max	.008	3	.004	3	.013	5	3.929e-4	5	NC	4	NC	2
348			min	-.008	2	-.003	2	-.008	1	-3.868e-4	1	2252.651	2	8253.711	5
349		4	max	.008	3	.005	2	.016	5	3.967e-4	5	NC	4	NC	2
350			min	-.008	2	-.004	3	-.009	1	-3.29e-4	1	1579.642	2	5165.92	5
351		5	max	.008	3	.013	2	.02	5	4.004e-4	5	NC	5	NC	2
352			min	-.008	2	-.011	3	-.009	1	-2.713e-4	1	1254.147	2	3671.374	5
353		6	max	.008	3	.019	2	.024	5	4.042e-4	5	NC	5	NC	2
354			min	-.008	2	-.017	3	-.009	1	-2.136e-4	1	1068.476	2	2804.779	5
355		7	max	.008	3	.024	2	.028	5	4.08e-4	5	NC	5	NC	2
356			min	-.008	2	-.021	3	-.008	1	-1.558e-4	1	954.316	2	2247.072	5
357		8	max	.008	3	.027	2	.032	5	4.117e-4	5	NC	5	NC	1
358			min	-.008	2	-.024	3	-.006	1	-9.812e-5	1	883.131	2	1862.726	5
359		9	max	.008	3	.03	2	.037	5	4.155e-4	5	NC	5	NC	1
360			min	-.008	2	-.026	3	-.005	1	-4.039e-5	1	841.472	2	1583.379	4
361		10	max	.008	3	.031	2	.041	5	4.249e-4	4	NC	5	NC	1
362			min	-.008	2	-.026	3	-.003	1	8.439e-6	10	822.958	2	1357.278	4
363		11	max	.008	3	.03	2	.046	4	4.415e-4	4	NC	5	NC	1
364			min	-.008	2	-.025	3	0	1	1.219e-5	10	825.348	2	1187.04	4
365		12	max	.008	3	.028	2	.051	4	4.58e-4	4	NC	5	NC	1
366			min	-.008	2	-.023	3	0	10	1.594e-5	10	849.693	2	1055.981	4
367		13	max	.008	3	.025	2	.056	4	4.746e-4	4	NC	5	NC	2
368			min	-.009	2	-.02	3	0	10	1.969e-5	10	900.873	2	953.39	4
369		14	max	.008	3	.02	2	.06	4	4.911e-4	4	NC	5	NC	2
370			min	-.009	2	-.015	3	0	10	2.344e-5	10	990.037	2	872.138	4
371		15	max	.008	3	.013	2	.065	4	5.077e-4	4	NC	4	NC	2
372			min	-.009	2	-.01	3	0	10	2.719e-5	10	1141.54	2	807.341	4
373		16	max	.008	3	.005	2	.069	4	7.589e-4	4	NC	4	NC	2
374			min	-.009	2	-.004	3	0	10	2.998e-5	10	1414.625	2	755.579	4
375		17	max	.008	3	.004	3	.072	4	6.594e-3	4	NC	4	NC	2
376			min	-.009	2	-.005	2	0	10	-1.297e-5	1	1996.329	2	714.469	4
377		18	max	.008	3	.012	3	.075	4	4.334e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	10	-1.969e-3	3	3852.882	2	682.192	4
379		19	max	.008	3	.02	3	.078	4	8.729e-3	2	NC	1	NC	1
380			min	-.009	2	-.03	2	-.002	1	-4.016e-3	3	NC	1	658.265	4
381	M5	1	max	.021	3	.071	3	.007	5	9.443e-6	4	NC	1	NC	1
382			min	-.025	2	-.065	2	-.004	1	3.427e-8	2	NC	1	NC	1
383		2	max	.021	3	.041	3	.01	5	1.952e-4	5	NC	4	NC	1
384			min	-.025	2	-.037	2	-.004	1	-6.538e-5	1	1597.916	2	NC	1



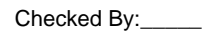
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.021	3	.013	3	.013	5	3.78e-4	5	NC	5	NC	1
386			min	-.025	2	-.01	1	-.003	1	-1.296e-4	1	818.586	2	NC	1
387		4	max	.021	3	.014	2	.017	5	3.946e-4	5	NC	5	NC	1
388			min	-.025	2	-.01	3	-.003	1	-1.234e-4	1	572.579	2	NC	1
389		5	max	.021	3	.034	2	.021	5	4.112e-4	5	NC	5	NC	1
390			min	-.025	2	-.03	3	-.003	1	-1.172e-4	1	453.631	2	NC	1
391		6	max	.021	3	.051	2	.025	5	4.278e-4	5	NC	5	NC	1
392			min	-.025	2	-.045	3	-.003	1	-1.109e-4	1	385.737	2	NC	1
393		7	max	.021	3	.065	2	.03	5	4.444e-4	5	NC	5	NC	1
394			min	-.025	2	-.056	3	-.003	1	-1.047e-4	1	343.92	2	NC	1
395		8	max	.021	3	.075	2	.034	5	4.61e-4	5	NC	5	NC	1
396			min	-.025	2	-.064	3	-.003	1	-9.851e-5	1	317.752	2	NC	1
397		9	max	.021	3	.082	2	.039	5	4.776e-4	5	NC	15	NC	1
398			min	-.025	2	-.069	3	-.003	1	-9.229e-5	1	302.312	2	NC	1
399		10	max	.021	3	.084	2	.044	5	4.942e-4	5	NC	15	NC	1
400			min	-.025	2	-.069	3	-.003	1	-8.608e-5	1	295.257	2	NC	1
401		11	max	.021	3	.083	2	.049	5	5.108e-4	5	NC	15	NC	1
402			min	-.025	2	-.067	3	-.003	1	-7.986e-5	1	295.75	2	NC	1
403		12	max	.021	3	.078	2	.053	5	5.274e-4	5	NC	5	NC	1
404			min	-.025	2	-.061	3	-.002	1	-7.365e-5	1	304.146	2	NC	1
405		13	max	.021	3	.068	2	.058	5	5.44e-4	5	NC	5	NC	1
406			min	-.025	2	-.053	3	-.002	1	-6.743e-5	1	322.178	2	NC	1
407		14	max	.02	3	.054	2	.062	4	5.606e-4	5	NC	5	NC	1
408			min	-.025	2	-.041	3	-.002	1	-6.122e-5	1	353.84	2	NC	1
409		15	max	.02	3	.035	2	.066	4	5.772e-4	5	NC	5	NC	1
410			min	-.025	2	-.027	3	-.002	1	-5.5e-5	1	407.887	2	NC	1
411		16	max	.02	3	.012	2	.07	4	8.251e-4	5	NC	5	NC	1
412			min	-.025	2	-.01	3	-.002	1	-5.275e-5	1	505.725	2	NC	1
413		17	max	.02	3	.01	3	.073	4	6.601e-3	4	NC	5	NC	1
414			min	-.025	2	-.016	2	-.002	1	-1.448e-4	1	715.763	2	NC	1
415		18	max	.02	3	.031	3	.076	4	3.387e-3	4	NC	4	NC	1
416			min	-.025	2	-.049	2	-.002	1	-7.396e-5	1	1389.66	2	NC	1
417		19	max	.02	3	.054	3	.078	4	3.197e-6	5	NC	1	NC	1
418			min	-.025	2	-.084	2	-.002	1	-3.761e-7	3	NC	1	NC	1
419	M9	1	max	.008	3	.025	3	.006	5	8.493e-3	3	NC	1	NC	1
420			min	-.008	2	-.023	2	-.004	1	-6.889e-3	1	NC	1	NC	1
421		2	max	.008	3	.014	3	.005	5	4.197e-3	3	NC	4	NC	1
422			min	-.008	2	-.013	2	0	1	-3.354e-3	1	4375.14	2	NC	1
423		3	max	.008	3	.004	3	.006	4	1.143e-4	1	NC	4	NC	1
424			min	-.008	2	-.003	2	0	3	-2.033e-5	3	2253.92	2	NC	1
425		4	max	.008	3	.005	2	.007	4	6.779e-5	1	NC	4	NC	1
426			min	-.008	2	-.004	3	-.001	3	-2.737e-5	3	1580.552	2	NC	1
427		5	max	.008	3	.013	2	.009	4	2.637e-5	2	NC	4	NC	1
428			min	-.008	2	-.011	3	-.002	3	-3.442e-5	3	1254.868	2	NC	1
429		6	max	.008	3	.019	2	.011	4	9.211e-6	2	NC	5	NC	1
430			min	-.008	2	-.017	3	-.002	3	-4.146e-5	3	1069.084	2	7603.449	4
431		7	max	.008	3	.024	2	.015	4	2.94e-6	10	NC	5	NC	1
432			min	-.008	2	-.021	3	-.003	3	-7.182e-5	1	954.85	2	5019.354	4
433		8	max	.008	3	.027	2	.018	4	-8.169e-7	10	NC	5	NC	1
434			min	-.008	2	-.024	3	-.003	3	-1.184e-4	1	883.616	2	3592.553	4
435		9	max	.008	3	.03	2	.022	5	-2.03e-6	15	NC	5	NC	1
436			min	-.008	2	-.026	3	-.003	3	-1.649e-4	1	841.925	2	2720.271	4
437		10	max	.008	3	.03	2	.027	5	-2.833e-7	15	NC	5	NC	1
438			min	-.008	2	-.026	3	-.004	3	-2.114e-4	1	823.391	2	2147.15	4
439		11	max	.008	3	.03	2	.033	5	1.829e-6	5	NC	5	NC	1
440			min	-.008	2	-.025	3	-.004	1	-2.58e-4	1	825.773	2	1749.859	4
441		12	max	.008	3	.028	2	.038	5	4.347e-6	5	NC	5	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.075	3	.033	1	6.624e-3	2	NC	5	NC	2
500		min	-.078	4	-.148	2	-.002	10	-4.401e-3	3	1164.078	2	3647.571	1
501	4	max	.002	1	.092	3	.049	1	7.549e-3	2	NC	5	NC	3
502		min	-.078	4	-.184	2	-.002	10	-4.979e-3	3	894.577	2	2534.537	1
503	5	max	.002	1	.1	3	.056	1	8.474e-3	2	NC	5	NC	3
504		min	-.078	4	-.198	2	-.002	10	-5.556e-3	3	820.083	2	2248.293	1
505	6	max	.002	1	.098	3	.051	1	9.399e-3	2	NC	5	NC	5
506		min	-.078	4	-.19	2	-.004	10	-6.134e-3	3	858.048	2	2442.766	1
507	7	max	.002	1	.088	3	.036	1	1.032e-2	2	NC	5	NC	2
508		min	-.078	4	-.166	2	-.007	10	-6.712e-3	3	1015.002	2	3379.758	1
509	8	max	.002	1	.074	3	.023	3	1.125e-2	2	NC	5	NC	2
510		min	-.078	4	-.131	2	-.011	2	-7.289e-3	3	1359.715	2	7412.788	1
511	9	max	.002	1	.06	3	.022	3	1.217e-2	2	NC	4	NC	1
512		min	-.078	4	-.099	2	-.021	2	-7.867e-3	3	1998.807	2	9709.524	3
513	10	max	.002	1	.054	3	.02	3	1.31e-2	2	NC	4	NC	1
514		min	-.078	4	-.084	2	-.025	2	-8.445e-3	3	2554.793	2	8276.096	2
515	11	max	.002	1	.06	3	.019	3	1.217e-2	2	NC	4	NC	1
516		min	-.078	4	-.099	2	-.021	2	-7.866e-3	3	1998.807	2	NC	1
517	12	max	.002	1	.074	3	.019	3	1.125e-2	2	NC	5	NC	2
518		min	-.078	4	-.131	2	-.011	2	-7.287e-3	3	1359.715	2	7352.92	1
519	13	max	.002	1	.088	3	.036	1	1.032e-2	2	NC	5	NC	2
520		min	-.078	4	-.166	2	-.007	10	-6.709e-3	3	1015.002	2	3375.409	1
521	14	max	.002	1	.098	3	.051	1	9.4e-3	2	NC	5	NC	3
522		min	-.078	4	-.19	2	-.004	10	-6.13e-3	3	858.048	2	2447.082	1
523	15	max	.002	1	.099	3	.056	1	8.475e-3	2	NC	5	NC	3
524		min	-.078	4	-.198	2	-.002	5	-5.552e-3	3	820.083	2	2257.773	1
525	16	max	.002	1	.092	3	.049	1	7.55e-3	2	NC	5	NC	3
526		min	-.078	4	-.184	2	-.005	5	-4.973e-3	3	894.577	2	2552.078	1
527	17	max	.002	1	.075	3	.032	1	6.625e-3	2	NC	5	NC	2
528		min	-.078	4	-.148	2	-.007	5	-4.395e-3	3	1164.078	2	3686.719	1
529	18	max	.002	1	.05	3	.011	1	5.701e-3	2	NC	4	NC	2
530		min	-.078	4	-.094	2	-.005	5	-3.816e-3	3	2133.538	2	8333.542	1
531	19	max	.002	1	.02	3	.008	3	4.776e-3	2	NC	1	NC	1
532		min	-.078	4	-.03	2	-.009	2	-3.238e-3	3	NC	1	NC	1
533	M15	max	0	1	0	1	0	1	3.693e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.601e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.007	4	8.232e-4	3	NC	1	NC	1
536		min	0	5	-.006	1	0	3	-5.805e-4	5	NC	1	NC	1
537	3	max	0	3	0	15	.014	4	1.277e-3	3	NC	5	NC	1
538		min	-.001	5	-.012	1	-.003	3	-1.021e-3	2	6575.021	2	5374.339	4
539	4	max	0	3	0	15	.022	4	1.731e-3	3	NC	5	NC	9
540		min	-.002	5	-.018	1	-.006	3	-1.49e-3	2	4510.848	2	3466.932	4
541	5	max	0	3	0	15	.03	4	2.185e-3	3	NC	5	NC	9
542		min	-.003	5	-.023	1	-.011	3	-1.959e-3	2	3519.861	2	2601.877	4
543	6	max	0	3	0	15	.036	4	2.639e-3	3	NC	5	9094.413	9
544		min	-.003	5	-.027	1	-.015	3	-2.428e-3	2	2962.335	2	2144.227	4
545	7	max	0	3	0	15	.041	4	3.092e-3	3	NC	5	7162.601	9
546		min	-.004	5	-.03	1	-.02	3	-2.897e-3	2	2627.057	2	1890.947	4
547	8	max	0	3	0	15	.044	4	3.546e-3	3	NC	5	5939.599	9
548		min	-.005	5	-.033	1	-.025	3	-3.367e-3	2	2425.839	2	1761.36	4
549	9	max	0	3	.001	15	.045	4	4.e-3	3	NC	5	5135.701	9
550		min	-.005	5	-.035	1	-.029	3	-3.836e-3	2	2317.532	2	1721.774	4
551	10	max	0	3	.001	5	.044	4	4.454e-3	3	NC	5	4604.241	9
552		min	-.006	5	-.035	1	-.033	3	-4.305e-3	2	2283.269	2	1730.392	3
553	11	max	0	3	.002	5	.041	4	4.908e-3	3	NC	5	4267.96	9
554		min	-.006	5	-.035	1	-.035	3	-4.774e-3	2	2317.532	2	1598.744	3
555	12	max	0	3	.002	5	.036	4	5.362e-3	3	NC	5	4088.816	9



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.007	5	-0.033	1	-0.036	3	-5.244e-3	2	2425.839	2	1527.406	3
557	13	max	0	3	.003	5	.034	1	5.816e-3	3	NC	5	4859.776	15
558		min	-0.008	5	-0.031	1	-0.035	3	-5.713e-3	2	2627.057	2	1511.85	3
559	14	max	0	3	.003	5	.031	1	6.269e-3	3	NC	5	6816.109	15
560		min	-0.008	5	-0.028	1	-0.032	3	-6.182e-3	2	2962.335	2	1558.614	3
561	15	max	.001	3	.003	5	.026	1	6.723e-3	3	NC	5	NC	15
562		min	-0.009	5	-0.024	1	-0.027	3	-6.651e-3	2	3519.861	2	1691.862	3
563	16	max	.001	3	.004	5	.019	1	7.177e-3	3	NC	5	NC	5
564		min	-.01	5	-0.019	1	-0.018	3	-7.121e-3	2	4510.848	2	1977.286	3
565	17	max	.001	3	.004	5	.009	1	7.631e-3	3	NC	5	NC	4
566		min	-.01	5	-0.014	1	-0.007	3	-7.59e-3	2	6575.021	2	2621.035	3
567	18	max	.001	3	.005	5	.007	3	8.085e-3	3	NC	1	NC	4
568		min	-0.011	5	-0.008	1	-.01	2	-8.059e-3	2	NC	1	4666.018	3
569	19	max	.001	3	.006	5	.025	3	8.539e-3	3	NC	1	NC	1
570		min	-0.012	5	-0.002	9	-0.027	2	-8.528e-3	2	NC	1	NC	1
571	M16A	1	max	0	10	0	.009	3	2.97e-3	3	NC	1	NC	1
572		min	-0.004	4	-0.003	4	-0.009	2	-2.981e-3	2	NC	1	NC	1
573	2	max	0	10	-0.003	12	.002	9	2.842e-3	3	NC	1	NC	1
574		min	-0.004	4	-0.014	4	-0.003	5	-2.842e-3	2	7556.517	4	NC	1
575	3	max	0	10	-0.006	12	.008	1	2.715e-3	3	NC	12	NC	4
576		min	-0.004	4	-0.023	4	-0.007	5	-2.703e-3	2	3845.253	4	5836.426	1
577	4	max	0	10	-0.009	12	.013	1	2.587e-3	3	9021.697	12	NC	10
578		min	-0.004	4	-0.032	4	-0.014	5	-2.564e-3	2	2638.068	4	4428.676	1
579	5	max	0	10	-0.011	12	.016	1	2.46e-3	3	7039.721	12	NC	14
580		min	-0.003	4	-0.041	4	-0.022	5	-2.424e-3	2	2058.512	4	3805.422	5
581	6	max	0	10	-0.013	12	.018	1	2.332e-3	3	5924.67	12	NC	14
582		min	-0.003	4	-0.048	4	-.03	5	-2.285e-3	2	1732.455	4	2730.139	5
583	7	max	0	10	-0.015	12	.018	1	2.205e-3	3	5254.113	12	8983.405	10
584		min	-0.003	4	-0.053	4	-0.037	5	-2.146e-3	2	1536.375	4	2158.012	5
585	8	max	0	10	-0.016	12	.018	1	2.077e-3	3	4851.678	12	9236.15	10
586		min	-0.003	4	-0.057	4	-0.044	5	-2.007e-3	2	1418.698	4	1830.409	5
587	9	max	0	10	-0.017	12	.017	1	1.949e-3	3	4635.064	12	9872.426	10
588		min	-0.002	4	-0.059	4	-0.048	5	-1.868e-3	2	1355.357	4	1642.008	5
589	10	max	0	10	-0.017	12	.016	1	1.822e-3	3	4566.538	12	NC	10
590		min	-0.002	4	-.06	4	-0.051	5	-1.729e-3	2	1335.319	4	1545.444	5
591	11	max	0	10	-0.017	12	.014	1	1.694e-3	3	4635.064	12	NC	10
592		min	-0.002	4	-0.059	4	-0.052	5	-1.59e-3	2	1355.357	4	1520.21	5
593	12	max	0	10	-0.016	12	.012	1	1.567e-3	3	4851.678	12	NC	9
594		min	-0.002	4	-0.056	4	-0.051	5	-1.451e-3	2	1418.698	4	1561.881	5
595	13	max	0	10	-0.015	12	.009	1	1.439e-3	3	5254.113	12	NC	9
596		min	-0.001	4	-0.052	4	-0.047	5	-1.311e-3	2	1536.375	4	1680.299	5
597	14	max	0	10	-0.013	12	.007	1	1.311e-3	3	5924.67	12	NC	2
598		min	-0.001	4	-0.046	4	-0.041	5	-1.172e-3	2	1732.455	4	1905.268	5
599	15	max	0	10	-0.011	12	.004	1	1.184e-3	3	7039.721	12	NC	1
600		min	0	4	-0.039	4	-0.034	5	-1.033e-3	2	2058.512	4	2306.484	5
601	16	max	0	10	-0.009	12	.002	1	1.056e-3	3	9021.697	12	NC	1
602		min	0	4	-.03	4	-0.026	5	-8.941e-4	2	2638.068	4	3057.614	5
603	17	max	0	10	-0.006	12	0	9	9.287e-4	3	NC	12	NC	1
604		min	0	4	-0.021	4	-0.017	5	-7.55e-4	2	3845.253	4	4697.957	5
605	18	max	0	10	-0.003	12	0	9	8.702e-4	4	NC	1	NC	1
606		min	0	4	-0.011	4	-0.008	5	-6.159e-4	2	7556.517	4	9980.006	5
607	19	max	0	1	0	1	0	1	9.418e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.767e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

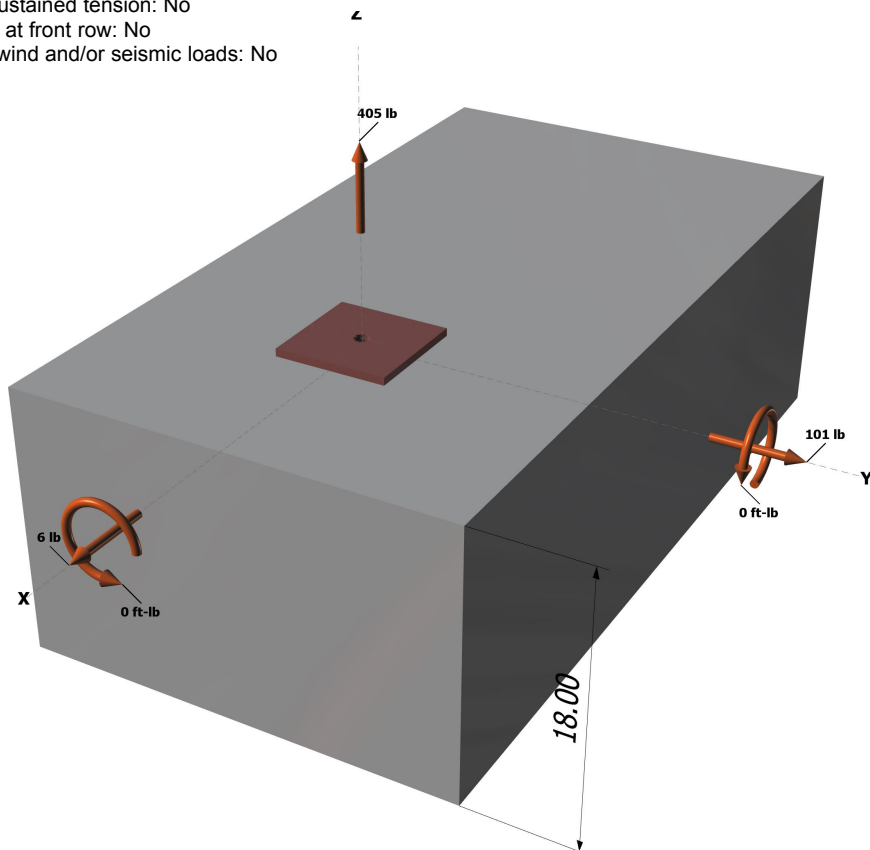
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



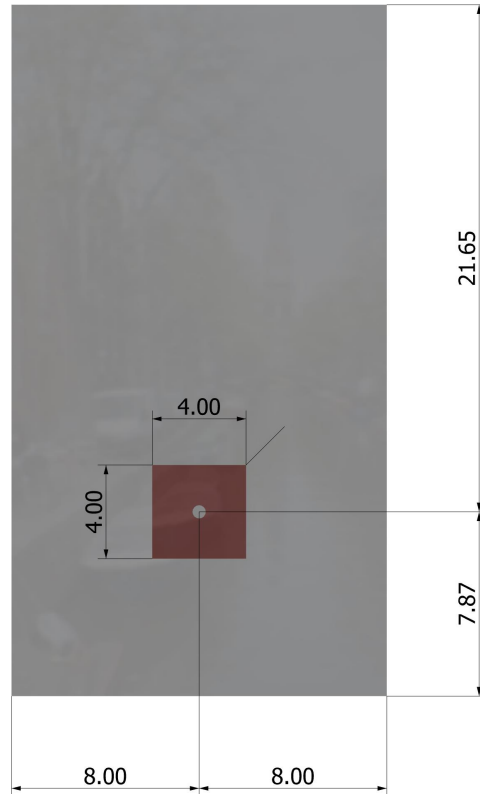
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

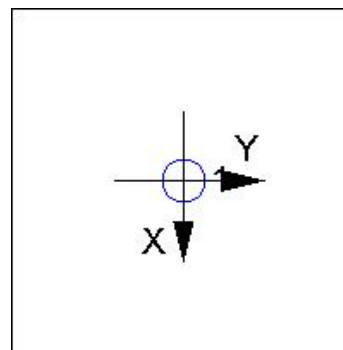
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

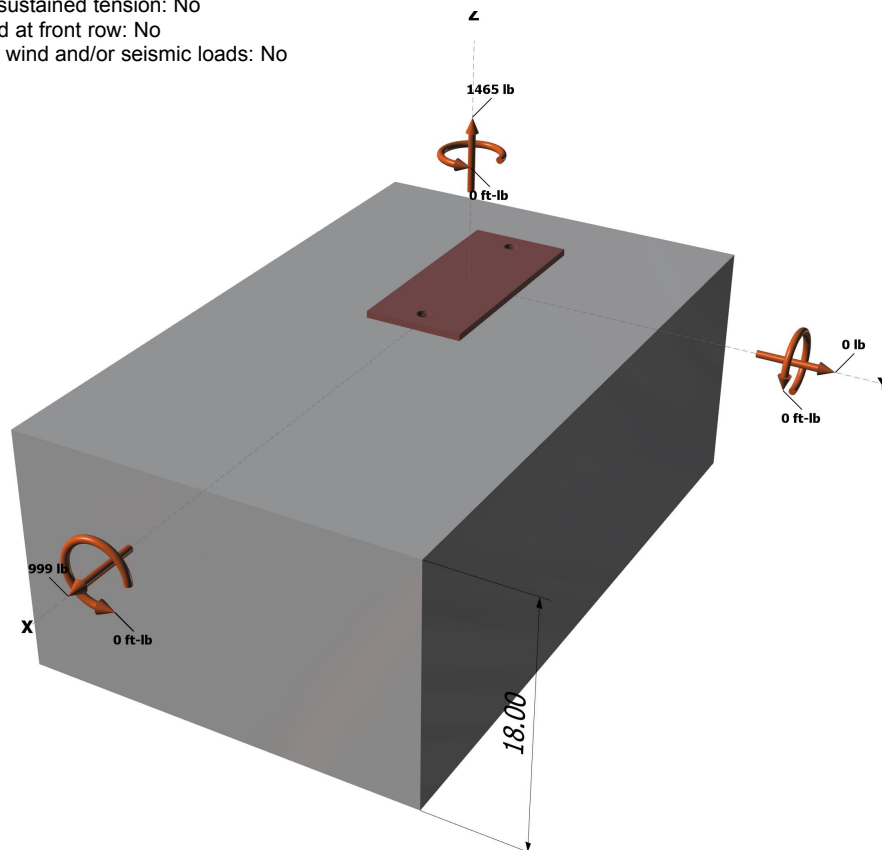
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



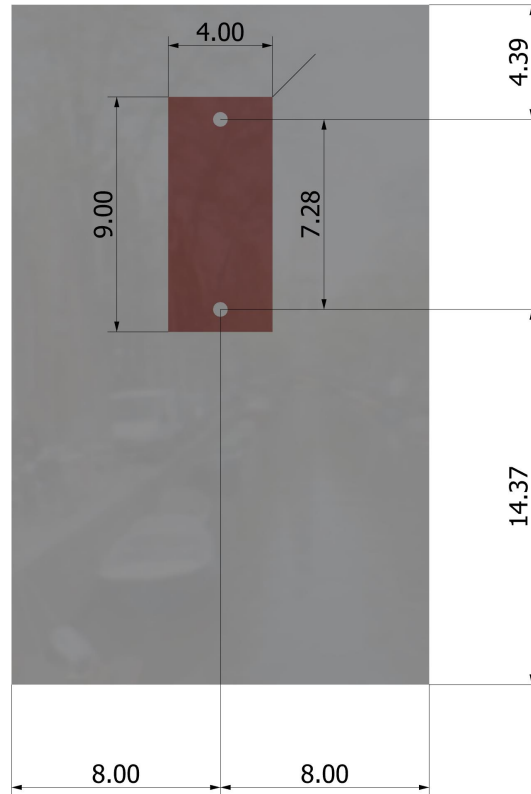
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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