



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.662 k-ft
$M_z$ =	-0.006 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>60%</b>

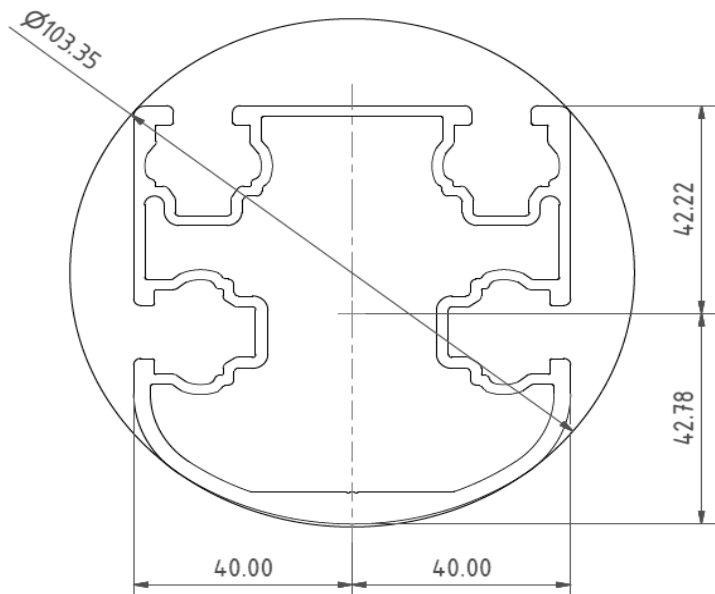


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.339 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.843 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>99%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.153 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>11%</u>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.013 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.049 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<u>35%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.013 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.287 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<b>25%</b>



DETAIL VIEW

### 5. FOUNDATION DESIGN CALCULATIONS

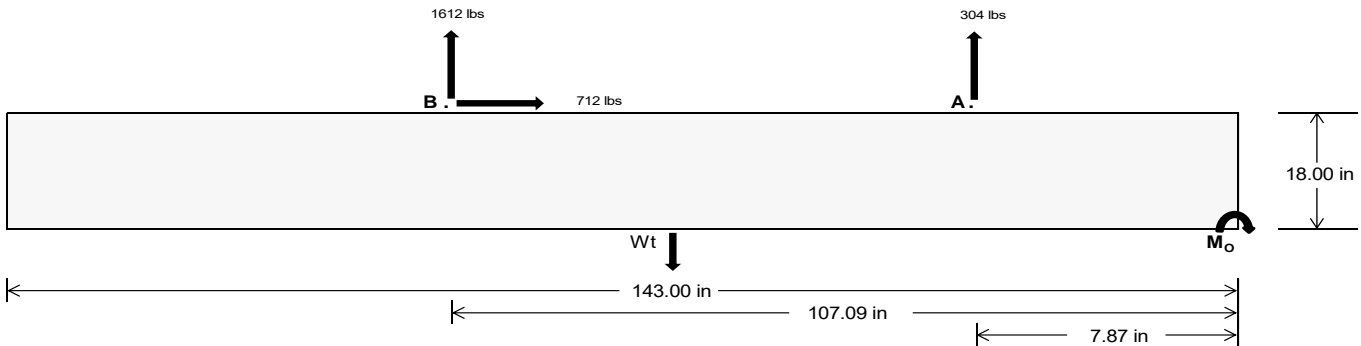
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<b>1273.28</b>	<b>6713.44</b> k
Compressive Load =		<b>4099.25</b>	<b>4928.78</b> k
Lateral Load =		<b>7.68</b>	<b>2961.79</b> k
Moment (Weak Axis) =		<b>0.02</b>	<b>0.00</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 187852.3$  in-lbs  
Resisting Force Required = 2627.30 lbs  
S.F. = 1.67  
Weight Required = 4378.84 lbs  
Minimum Width = 36 in  
Weight Provided = 7775.63 lbs

### Sliding

Force = 712.24 lbs  
Friction = 0.4  
Weight Required = 1780.61 lbs  
Resisting Weight = 7775.63 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 712.24 lbs  
Cohesion = 130 psf  
Area = 35.75 ft<sup>2</sup>  
Resisting = 3887.81 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 36in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 36in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(3 \text{ ft}) =$

Ballast Width	36 in	37 in	38 in	39 in
	7776 lbs	7992 lbs	8208 lbs	8424 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in
$F_A$	1135 lbs	1135 lbs	1135 lbs	1135 lbs	1723 lbs	1723 lbs	1723 lbs	1723 lbs	2048 lbs	2048 lbs	2048 lbs	2048 lbs	-608 lbs	-608 lbs	-608 lbs	-608 lbs
$F_B$	1201 lbs	1201 lbs	1201 lbs	1201 lbs	2106 lbs	2106 lbs	2106 lbs	2106 lbs	2380 lbs	2380 lbs	2380 lbs	2380 lbs	-3224 lbs	-3224 lbs	-3224 lbs	-3224 lbs
$F_V$	99 lbs	99 lbs	99 lbs	99 lbs	1252 lbs	1252 lbs	1252 lbs	1252 lbs	1006 lbs	1006 lbs	1006 lbs	1006 lbs	-1424 lbs	-1424 lbs	-1424 lbs	-1424 lbs
$P_{total}$	10112 lbs	10328 lbs	10544 lbs	10760 lbs	11605 lbs	11821 lbs	12037 lbs	12253 lbs	12204 lbs	12420 lbs	12636 lbs	12852 lbs	833 lbs	963 lbs	1092 lbs	1222 lbs
$M$	2603 lbs-ft	2603 lbs-ft	2603 lbs-ft	2603 lbs-ft	4770 lbs-ft	4770 lbs-ft	4770 lbs-ft	4770 lbs-ft	5310 lbs-ft	5310 lbs-ft	5310 lbs-ft	5310 lbs-ft	4202 lbs-ft	4202 lbs-ft	4202 lbs-ft	4202 lbs-ft
$e$	0.26 ft	0.25 ft	0.25 ft	0.24 ft	0.41 ft	0.40 ft	0.40 ft	0.39 ft	0.44 ft	0.43 ft	0.42 ft	0.41 ft	5.04 ft	4.36 ft	3.85 ft	3.44 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	246.2 psf	245.4 psf	244.7 psf	244.0 psf	257.4 psf	256.3 psf	255.3 psf	254.3 psf	266.6 psf	265.3 psf	264.0 psf	262.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	319.5 psf	316.8 psf	314.1 psf	311.7 psf	391.8 psf	387.1 psf	382.6 psf	378.4 psf	416.2 psf	410.8 psf	405.7 psf	400.9 psf	202.3 psf	130.6 psf	108.9 psf	99.5 psf

Maximum Bearing Pressure = 416 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

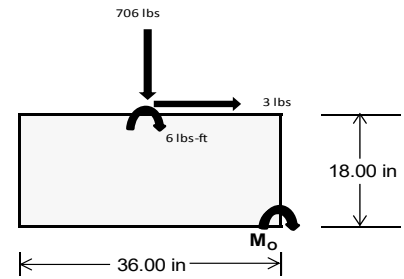
#### Overturning Check

$M_o = 1048.6 \text{ ft-lbs}$   
 Resisting Force Required = 699.05 lbs  
 S.F. = 1.67  
 Weight Required = 1165.08 lbs  
 Minimum Width = **36 in**  
 Weight Provided = 7775.63 lbs

*A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	36 in			36 in			36 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	206 lbs	485 lbs	206 lbs	706 lbs	1920 lbs	706 lbs	60 lbs	142 lbs	60 lbs
$F_v$	1 lbs	0 lbs	1 lbs	3 lbs	0 lbs	3 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9832 lbs	7776 lbs	9832 lbs	9870 lbs	7776 lbs	9870 lbs	2875 lbs	7776 lbs	2875 lbs
$M$	3 lbs-ft	0 lbs-ft	3 lbs-ft	11 lbs-ft	0 lbs-ft	11 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft
$f_{min}$	274.8 psf	217.5 psf	274.8 psf	275.5 psf	217.5 psf	275.5 psf	80.4 psf	217.5 psf	80.4 psf
$f_{max}$	275.2 psf	217.5 psf	275.2 psf	276.7 psf	217.5 psf	276.7 psf	80.4 psf	217.5 psf	80.4 psf



Maximum Bearing Pressure = 277 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.**

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	1.134 k
Allowable Uplift =	1.214 k
Utilization =	<u>93%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.637 k
Allowable Uplift =	4.357 k
Utilization =	<u>61%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.153 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>42%</u>

#### Rear Strut

Maximum Axial Load =	4.625 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>62%</u>

#### Diagonal Strut

Maximum Axial Load =	2.269 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>31%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 78 \text{ in}$$

$$J = 0.432$$

$$215.785$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 78$$

$$J = 0.432$$

$$137.226$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.41345$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77788$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 13.6277 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-91.409	-91.409	0	0
2	M14	y	-91.409	-91.409	0	0
3	M15	y	-143.642	-143.642	0	0
4	M16	y	-143.642	-143.642	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	208.934	208.934	0	0
2	M14	y	160.183	160.183	0	0
3	M15	y	87.056	87.056	0	0
4	M16	y	87.056	87.056	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes	Y		1	1.2	3	1.6	4	.8										
2	LRFD 1.2D + 1.6W + 0.5S	Yes	Y		1	1.2	3	.5	4	1.6										
3	LRFD 0.9D + 1.6W	Yes	Y		2	.9					5	1.6								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 20° 130mph 30psf 6.5ft 7-05 NS.r3d] Page 19



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	36.371	1	235.543	2	2.979	3	.011	2	-.003	15	.79	3
28			min	1.353	15	-374.263	3	-23.521	1	0	15	-.071	1	-.381	2
29		15	max	36.371	1	99.104	2	4.965	1	.011	2	-.003	12	.974	3
30			min	1.353	15	-137.078	3	-1.2	10	0	15	-.078	1	-.502	2
31		16	max	36.371	1	100.106	3	33.451	1	.011	2	0	3	.988	3
32			min	1.353	15	-38.046	1	1.244	15	0	15	-.064	1	-.524	2
33		17	max	36.371	1	337.291	3	61.937	1	.011	2	.005	3	.83	3
34			min	1.353	15	-173.774	2	2.259	15	0	15	-.03	1	-.448	2
35		18	max	36.371	1	574.476	3	90.422	1	.011	2	.025	1	.501	3
36			min	1.353	15	-310.213	2	3.273	15	0	15	-.001	10	-.273	2
37		19	max	36.371	1	811.661	3	118.908	1	.011	2	.101	1	0	1
38			min	1.353	15	-446.652	2	4.288	15	0	15	.004	15	0	3
39	M14	1	max	26.11	1	551.953	2	-4.501	15	.016	3	.126	1	0	1
40			min	.964	15	-681.306	3	-124.772	1	-.017	2	.005	15	0	3
41		2	max	26.11	1	415.514	2	-3.486	15	.016	3	.046	1	.427	3
42			min	.964	15	-500.456	3	-96.286	1	-.017	2	0	10	-.349	2
43		3	max	26.11	1	279.075	2	-2.471	15	.016	3	.007	3	.723	3
44			min	.964	15	-319.606	3	-67.8	1	-.017	2	-.013	1	-.6	2
45		4	max	26.11	1	142.636	2	-1.457	15	.016	3	.002	3	.888	3
46			min	.964	15	-138.755	3	-39.315	1	-.017	2	-.052	1	-.752	2
47		5	max	26.11	1	42.095	3	.746	10	.016	3	-.002	12	.923	3
48			min	.964	15	-3.033	9	-10.829	1	-.017	2	-.07	1	-.806	2
49		6	max	26.11	1	222.945	3	17.657	1	.016	3	-.002	15	.828	3
50			min	.964	15	-131.71	1	-3.478	3	-.017	2	-.067	1	-.761	2
51		7	max	26.11	1	403.796	3	46.143	1	.016	3	-.002	15	.601	3
52			min	.964	15	-266.681	2	-1.931	3	-.017	2	-.044	1	-.618	2
53		8	max	26.11	1	584.646	3	74.629	1	.016	3	.004	2	.244	3
54			min	.964	15	-403.12	2	-.384	3	-.017	2	-.008	3	-.376	2
55		9	max	26.11	1	765.496	3	103.115	1	.016	3	.063	1	.018	9
56			min	.964	15	-539.559	2	.951	12	-.017	2	-.008	3	-.243	3
57		10	max	26.11	1	675.998	2	-1.982	12	.017	2	.148	1	.43	1
58			min	.964	15	-946.347	3	-131.601	1	-.016	3	-.007	3	-.861	3
59		11	max	26.11	1	539.559	2	-.951	12	.017	2	.063	1	.018	9
60			min	.964	15	-765.496	3	-103.115	1	-.016	3	-.008	3	-.243	3
61		12	max	26.11	1	403.12	2	.384	3	.017	2	.004	2	.244	3
62			min	.964	15	-584.646	3	-74.629	1	-.016	3	-.008	3	-.376	2
63		13	max	26.11	1	266.681	2	1.931	3	.017	2	-.002	15	.601	3
64			min	.964	15	-403.796	3	-46.143	1	-.016	3	-.044	1	-.618	2
65		14	max	26.11	1	131.71	1	3.478	3	.017	2	-.002	15	.828	3
66			min	.964	15	-222.945	3	-17.657	1	-.016	3	-.067	1	-.761	2
67		15	max	26.11	1	3.033	9	10.829	1	.017	2	-.002	12	.923	3
68			min	.964	15	-42.095	3	-.746	10	-.016	3	-.07	1	-.806	2
69		16	max	26.11	1	138.755	3	39.315	1	.017	2	.002	3	.888	3
70			min	.964	15	-142.636	2	1.457	15	-.016	3	-.052	1	-.752	2
71		17	max	26.11	1	319.606	3	67.8	1	.017	2	.007	3	.723	3
72			min	.964	15	-279.075	2	2.471	15	-.016	3	-.013	1	-.6	2
73		18	max	26.11	1	500.456	3	96.286	1	.017	2	.046	1	.427	3
74			min	.964	15	-415.514	2	3.486	15	-.016	3	0	10	-.349	2
75		19	max	26.11	1	681.306	3	124.772	1	.017	2	.126	1	0	1
76			min	.964	15	-551.953	2	4.501	15	-.016	3	.005	15	0	3
77	M15	1	max	-1.011	15	756.022	2	-4.499	15	.018	2	.126	1	0	2
78			min	-27.096	1	-397.653	3	-124.827	1	-.013	3	.005	15	0	3
79		2	max	-1.011	15	559.224	2	-3.484	15	.018	2	.046	1	.252	3
80			min	-27.096	1	-301.305	3	-96.341	1	-.013	3	0	10	-.475	2
81		3	max	-1.011	15	362.427	2	-2.47	15	.018	2	.006	3	.435	3
82			min	-27.096	1	-204.957	3	-67.855	1	-.013	3	-.013	1	-.808	2
83		4	max	-1.011	15	165.63	2	-1.455	15	.018	2	.001	3	.548	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-27.096	1	-108.609	3	-39.369	1	-.013	3	-.052	1	-.998	2
85		5	max	-1.011	15	-.169	15	.626	10	.018	2	-.002	12	.592	3
86			min	-27.096	1	-31.167	2	-10.883	1	-.013	3	-.07	1	-1.047	2
87		6	max	-1.011	15	84.087	3	17.602	1	.018	2	-.002	15	.566	3
88			min	-27.096	1	-227.964	2	-3.104	3	-.013	3	-.067	1	-.953	2
89		7	max	-1.011	15	180.435	3	46.088	1	.018	2	-.002	15	.471	3
90			min	-27.096	1	-424.761	2	-1.557	3	-.013	3	-.044	1	-.718	2
91		8	max	-1.011	15	276.783	3	74.574	1	.018	2	.004	2	.306	3
92			min	-27.096	1	-621.558	2	-.01	3	-.013	3	-.008	3	-.34	2
93		9	max	-1.011	15	373.131	3	103.06	1	.018	2	.063	1	.18	2
94			min	-27.096	1	-818.356	2	1.185	12	-.013	3	-.007	3	0	15
95		10	max	-1.011	15	1015.153	2	-2.217	12	.013	3	.148	1	.842	2
96			min	-27.096	1	-469.479	3	-131.546	1	-.018	2	-.005	3	-.233	3
97		11	max	-1.011	15	818.356	2	-1.185	12	.013	3	.063	1	.18	2
98			min	-27.096	1	-373.131	3	-103.06	1	-.018	2	-.007	3	0	15
99		12	max	-1.011	15	621.558	2	.01	3	.013	3	.004	2	.306	3
100			min	-27.096	1	-276.783	3	-74.574	1	-.018	2	-.008	3	-.34	2
101		13	max	-1.011	15	424.761	2	1.557	3	.013	3	-.002	15	.471	3
102			min	-27.096	1	-180.435	3	-46.088	1	-.018	2	-.044	1	-.718	2
103		14	max	-1.011	15	227.964	2	3.104	3	.013	3	-.002	15	.566	3
104			min	-27.096	1	-84.087	3	-17.602	1	-.018	2	-.067	1	-.953	2
105		15	max	-1.011	15	31.167	2	10.883	1	.013	3	-.002	12	.592	3
106			min	-27.096	1	-.169	15	-.626	10	-.018	2	-.07	1	-1.047	2
107		16	max	-1.011	15	108.609	3	39.369	1	.013	3	.001	3	.548	3
108			min	-27.096	1	-165.63	2	1.455	15	-.018	2	-.052	1	-.998	2
109		17	max	-1.011	15	204.957	3	67.855	1	.013	3	.006	3	.435	3
110			min	-27.096	1	-362.427	2	2.47	15	-.018	2	-.013	1	-.808	2
111		18	max	-1.011	15	301.305	3	96.341	1	.013	3	.046	1	.252	3
112			min	-27.096	1	-559.224	2	3.484	15	-.018	2	0	10	-.475	2
113		19	max	-1.011	15	397.653	3	124.827	1	.013	3	.126	1	0	2
114			min	-27.096	1	-756.022	2	4.499	15	-.018	2	.005	15	0	3
115	M16	1	max	-1.51	15	657.258	2	-4.301	15	.005	1	.103	1	0	2
116			min	-40.828	1	-310.779	3	-119.58	1	-.011	3	.004	15	0	3
117		2	max	-1.51	15	460.461	2	-3.286	15	.005	1	.027	1	.19	3
118			min	-40.828	1	-214.431	3	-91.095	1	-.011	3	0	10	-.404	2
119		3	max	-1.51	15	263.664	2	-2.272	15	.005	1	.003	3	.31	3
120			min	-40.828	1	-118.083	3	-62.609	1	-.011	3	-.028	1	-.665	2
121		4	max	-1.51	15	66.867	2	-1.257	15	.005	1	0	12	.36	3
122			min	-40.828	1	-21.735	3	-34.123	1	-.011	3	-.063	1	-.784	2
123		5	max	-1.51	15	74.613	3	.774	10	.005	1	-.003	12	.341	3
124			min	-40.828	1	-129.93	2	-5.637	1	-.011	3	-.078	1	-.762	2
125		6	max	-1.51	15	170.961	3	22.849	1	.005	1	-.003	15	.252	3
126			min	-40.828	1	-326.728	2	-1.812	3	-.011	3	-.071	1	-.597	2
127		7	max	-1.51	15	267.309	3	51.335	1	.005	1	-.002	15	.094	3
128			min	-40.828	1	-523.525	2	-.265	3	-.011	3	-.045	1	-.29	2
129		8	max	-1.51	15	363.657	3	79.821	1	.005	1	.005	2	.159	2
130			min	-40.828	1	-720.322	2	.975	12	-.011	3	-.006	3	-.134	3
131		9	max	-1.51	15	460.005	3	108.306	1	.005	1	.071	1	.751	2
132			min	-40.828	1	-917.119	2	2.007	12	-.011	3	-.005	3	-.431	3
133		10	max	-1.51	15	1113.916	2	-3.038	12	.011	3	.159	1	1.484	2
134			min	-40.828	1	-556.354	3	-136.792	1	-.005	1	-.002	3	-.798	3
135		11	max	-1.51	15	917.119	2	-2.007	12	.011	3	.071	1	.751	2
136			min	-40.828	1	-460.005	3	-108.306	1	-.005	1	-.005	3	-.431	3
137		12	max	-1.51	15	720.322	2	-.975	12	.011	3	.005	2	.159	2
138			min	-40.828	1	-363.657	3	-79.821	1	-.005	1	-.006	3	-.134	3
139		13	max	-1.51	15	523.525	2	.265	3	.011	3	-.002	15	.094	3
140			min	-40.828	1	-267.309	3	-51.335	1	-.005	1	-.045	1	-.29	2



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-1.51	15	326.728	2	1.812	3	.011	3	-.003	15	.252	3
142			min	-40.828	1	-170.961	3	-22.849	1	-.005	1	-.071	1	-.597	2
143		15	max	-1.51	15	129.93	2	5.637	1	.011	3	-.003	12	.341	3
144			min	-40.828	1	-74.613	3	-.774	10	-.005	1	-.078	1	-.762	2
145		16	max	-1.51	15	21.735	3	34.123	1	.011	3	0	12	.36	3
146			min	-40.828	1	-66.867	2	1.257	15	-.005	1	-.063	1	-.784	2
147		17	max	-1.51	15	118.083	3	62.609	1	.011	3	.003	3	.31	3
148			min	-40.828	1	-263.664	2	2.272	15	-.005	1	-.028	1	-.665	2
149		18	max	-1.51	15	214.431	3	91.095	1	.011	3	.027	1	.19	3
150			min	-40.828	1	-460.461	2	3.286	15	-.005	1	0	10	-.404	2
151		19	max	-1.51	15	310.779	3	119.58	1	.011	3	.103	1	0	2
152			min	-40.828	1	-657.258	2	4.301	15	-.005	1	.004	15	0	3
153	M2	1	max	1122.477	2	2.159	4	.425	1	0	5	0	3	0	1
154			min	-1529.874	3	.507	15	.016	15	0	1	0	2	0	1
155		2	max	1122.893	2	2.15	4	.425	1	0	5	0	1	0	15
156			min	-1529.562	3	.505	15	.016	15	0	1	0	10	0	4
157		3	max	1123.309	2	2.141	4	.425	1	0	5	0	1	0	15
158			min	-1529.25	3	.503	15	.016	15	0	1	0	10	-.001	4
159		4	max	1123.725	2	2.133	4	.425	1	0	5	0	1	0	15
160			min	-1528.938	3	.501	15	.016	15	0	1	0	15	-.002	4
161		5	max	1124.141	2	2.124	4	.425	1	0	5	0	1	0	15
162			min	-1528.626	3	.499	15	.016	15	0	1	0	15	-.002	4
163		6	max	1124.556	2	2.115	4	.425	1	0	5	0	1	0	15
164			min	-1528.314	3	.497	15	.016	15	0	1	0	15	-.003	4
165		7	max	1124.972	2	2.107	4	.425	1	0	5	0	1	0	15
166			min	-1528.002	3	.495	15	.016	15	0	1	0	15	-.004	4
167		8	max	1125.388	2	2.098	4	.425	1	0	5	0	1	0	15
168			min	-1527.69	3	.493	15	.016	15	0	1	0	15	-.004	4
169		9	max	1125.804	2	2.089	4	.425	1	0	5	0	1	-.001	15
170			min	-1527.378	3	.491	15	.016	15	0	1	0	15	-.005	4
171		10	max	1126.22	2	2.08	4	.425	1	0	5	.001	1	-.001	15
172			min	-1527.066	3	.489	15	.016	15	0	1	0	15	-.005	4
173		11	max	1126.636	2	2.072	4	.425	1	0	5	.001	1	-.001	15
174			min	-1526.754	3	.487	15	.016	15	0	1	0	15	-.006	4
175		12	max	1127.052	2	2.063	4	.425	1	0	5	.001	1	-.002	15
176			min	-1526.443	3	.485	15	.016	15	0	1	0	15	-.007	4
177		13	max	1127.468	2	2.054	4	.425	1	0	5	.001	1	-.002	15
178			min	-1526.131	3	.483	15	.016	15	0	1	0	15	-.007	4
179		14	max	1127.883	2	2.046	4	.425	1	0	5	.002	1	-.002	15
180			min	-1525.819	3	.481	15	.016	15	0	1	0	15	-.008	4
181		15	max	1128.299	2	2.037	4	.425	1	0	5	.002	1	-.002	15
182			min	-1525.507	3	.479	15	.016	15	0	1	0	15	-.008	4
183		16	max	1128.715	2	2.028	4	.425	1	0	5	.002	1	-.002	15
184			min	-1525.195	3	.477	15	.016	15	0	1	0	15	-.009	4
185		17	max	1129.131	2	2.019	4	.425	1	0	5	.002	1	-.002	15
186			min	-1524.883	3	.475	15	.016	15	0	1	0	15	-.009	4
187		18	max	1129.547	2	2.011	4	.425	1	0	5	.002	1	-.002	15
188			min	-1524.571	3	.473	15	.016	15	0	1	0	15	-.01	4
189		19	max	1129.963	2	2.002	4	.425	1	0	5	.002	1	-.002	15
190			min	-1524.259	3	.47	15	.016	15	0	1	0	15	-.01	4
191	M3	1	max	654.331	2	9.102	4	.11	1	0	3	0	1	.01	4
192			min	-790.451	3	2.139	15	.004	15	0	1	0	15	.002	15
193		2	max	654.161	2	8.228	4	.11	1	0	3	0	1	.007	2
194			min	-790.579	3	1.934	15	.004	15	0	1	0	15	.001	12
195		3	max	653.99	2	7.353	4	.11	1	0	3	0	1	.004	2
196			min	-790.706	3	1.728	15	.004	15	0	1	0	15	0	3
197		4	max	653.82	2	6.479	4	.11	1	0	3	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-790.834	3	1.523	15	.004	15	0	1	0	15	-.002	3
199	5	max	653.65	2	5.604	4	.11	1	0	3	0	1	0	15
200		min	-790.962	3	1.317	15	.004	15	0	1	0	15	-.004	3
201	6	max	653.479	2	4.73	4	.11	1	0	3	0	1	-.001	15
202		min	-791.09	3	1.112	15	.004	15	0	1	0	15	-.006	4
203	7	max	653.309	2	3.855	4	.11	1	0	3	0	1	-.002	15
204		min	-791.217	3	.906	15	.004	15	0	1	0	15	-.008	4
205	8	max	653.139	2	2.981	4	.11	1	0	3	0	1	-.002	15
206		min	-791.345	3	.701	15	.004	15	0	1	0	15	-.01	4
207	9	max	652.968	2	2.106	4	.11	1	0	3	0	1	-.003	15
208		min	-791.473	3	.495	15	.004	15	0	1	0	15	-.011	4
209	10	max	652.798	2	1.232	4	.11	1	0	3	0	1	-.003	15
210		min	-791.601	3	.29	15	.004	15	0	1	0	15	-.012	4
211	11	max	652.628	2	.468	2	.11	1	0	3	0	1	-.003	15
212		min	-791.728	3	-.052	3	.004	15	0	1	0	15	-.012	4
213	12	max	652.457	2	-.122	15	.11	1	0	3	0	1	-.003	15
214		min	-791.856	3	-.563	3	.004	15	0	1	0	15	-.012	4
215	13	max	652.287	2	-.327	15	.11	1	0	3	0	1	-.003	15
216		min	-791.984	3	-1.391	4	.004	15	0	1	0	15	-.011	4
217	14	max	652.117	2	-.533	15	.11	1	0	3	0	1	-.002	15
218		min	-792.112	3	-2.266	4	.004	15	0	1	0	15	-.011	4
219	15	max	651.946	2	-.738	15	.11	1	0	3	0	1	-.002	15
220		min	-792.239	3	-3.14	4	.004	15	0	1	0	15	-.009	4
221	16	max	651.776	2	-.944	15	.11	1	0	3	0	1	-.002	15
222		min	-792.367	3	-4.015	4	.004	15	0	1	0	15	-.008	4
223	17	max	651.606	2	-1.149	15	.11	1	0	3	0	1	-.001	15
224		min	-792.495	3	-4.889	4	.004	15	0	1	0	15	-.005	4
225	18	max	651.435	2	-1.355	15	.11	1	0	3	0	1	0	15
226		min	-792.623	3	-5.764	4	.004	15	0	1	0	15	-.003	4
227	19	max	651.265	2	-1.56	15	.11	1	0	3	0	1	0	1
228		min	-792.75	3	-6.638	4	.004	15	0	1	0	15	0	1
229	M4	1	max	1083.946	2	0	1	-.224	15	0	1	0	1	0
230		min	-305.463	3	0	1	-6.101	1	0	1	0	15	0	1
231	2	max	1084.116	2	0	1	-.224	15	0	1	0	12	0	1
232		min	-305.335	3	0	1	-6.101	1	0	1	0	1	0	1
233	3	max	1084.286	2	0	1	-.224	15	0	1	0	15	0	1
234		min	-305.207	3	0	1	-6.101	1	0	1	0	1	0	1
235	4	max	1084.457	2	0	1	-.224	15	0	1	0	15	0	1
236		min	-305.079	3	0	1	-6.101	1	0	1	-.002	1	0	1
237	5	max	1084.627	2	0	1	-.224	15	0	1	0	15	0	1
238		min	-304.952	3	0	1	-6.101	1	0	1	-.002	1	0	1
239	6	max	1084.797	2	0	1	-.224	15	0	1	0	15	0	1
240		min	-304.824	3	0	1	-6.101	1	0	1	-.003	1	0	1
241	7	max	1084.968	2	0	1	-.224	15	0	1	0	15	0	1
242		min	-304.696	3	0	1	-6.101	1	0	1	-.004	1	0	1
243	8	max	1085.138	2	0	1	-.224	15	0	1	0	15	0	1
244		min	-304.568	3	0	1	-6.101	1	0	1	-.004	1	0	1
245	9	max	1085.308	2	0	1	-.224	15	0	1	0	15	0	1
246		min	-304.44	3	0	1	-6.101	1	0	1	-.005	1	0	1
247	10	max	1085.479	2	0	1	-.224	15	0	1	0	15	0	1
248		min	-304.313	3	0	1	-6.101	1	0	1	-.006	1	0	1
249	11	max	1085.649	2	0	1	-.224	15	0	1	0	15	0	1
250		min	-304.185	3	0	1	-6.101	1	0	1	-.006	1	0	1
251	12	max	1085.819	2	0	1	-.224	15	0	1	0	15	0	1
252		min	-304.057	3	0	1	-6.101	1	0	1	-.007	1	0	1
253	13	max	1085.99	2	0	1	-.224	15	0	1	0	15	0	1
254		min	-303.929	3	0	1	-6.101	1	0	1	-.008	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1086.16	2	0	1	-.224	15	0	1	0	15	0	1
256		min	-303.802	3	0	1	-6.101	1	0	1	-.009	1	0	1
257	15	max	1086.33	2	0	1	-.224	15	0	1	0	15	0	1
258		min	-303.674	3	0	1	-6.101	1	0	1	-.009	1	0	1
259	16	max	1086.501	2	0	1	-.224	15	0	1	0	15	0	1
260		min	-303.546	3	0	1	-6.101	1	0	1	-.01	1	0	1
261	17	max	1086.671	2	0	1	-.224	15	0	1	0	15	0	1
262		min	-303.418	3	0	1	-6.101	1	0	1	-.011	1	0	1
263	18	max	1086.842	2	0	1	-.224	15	0	1	0	15	0	1
264		min	-303.291	3	0	1	-6.101	1	0	1	-.011	1	0	1
265	19	max	1087.012	2	0	1	-.224	15	0	1	0	15	0	1
266		min	-303.163	3	0	1	-6.101	1	0	1	-.012	1	0	1
267	M6	1	max	3279.948	2	2.672	2	0	1	0	0	1	0	1
268		min	-4625.309	3	-.021	3	0	1	0	1	0	1	0	1
269	2	max	3280.364	2	2.665	2	0	1	0	1	0	1	0	3
270		min	-4624.997	3	-.026	3	0	1	0	1	0	1	0	2
271	3	max	3280.779	2	2.658	2	0	1	0	1	0	1	0	3
272		min	-4624.685	3	-.031	3	0	1	0	1	0	1	-.001	2
273	4	max	3281.195	2	2.652	2	0	1	0	1	0	1	0	3
274		min	-4624.373	3	-.036	3	0	1	0	1	0	1	-.002	2
275	5	max	3281.611	2	2.645	2	0	1	0	1	0	1	0	3
276		min	-4624.061	3	-.041	3	0	1	0	1	0	1	-.003	2
277	6	max	3282.027	2	2.638	2	0	1	0	1	0	1	0	3
278		min	-4623.749	3	-.046	3	0	1	0	1	0	1	-.004	2
279	7	max	3282.443	2	2.631	2	0	1	0	1	0	1	0	3
280		min	-4623.437	3	-.052	3	0	1	0	1	0	1	-.004	2
281	8	max	3282.859	2	2.625	2	0	1	0	1	0	1	0	3
282		min	-4623.125	3	-.057	3	0	1	0	1	0	1	-.005	2
283	9	max	3283.275	2	2.618	2	0	1	0	1	0	1	0	3
284		min	-4622.813	3	-.062	3	0	1	0	1	0	1	-.006	2
285	10	max	3283.691	2	2.611	2	0	1	0	1	0	1	0	3
286		min	-4622.502	3	-.067	3	0	1	0	1	0	1	-.007	2
287	11	max	3284.107	2	2.604	2	0	1	0	1	0	1	0	3
288		min	-4622.19	3	-.072	3	0	1	0	1	0	1	-.007	2
289	12	max	3284.522	2	2.597	2	0	1	0	1	0	1	0	3
290		min	-4621.878	3	-.077	3	0	1	0	1	0	1	-.008	2
291	13	max	3284.938	2	2.591	2	0	1	0	1	0	1	0	3
292		min	-4621.566	3	-.082	3	0	1	0	1	0	1	-.009	2
293	14	max	3285.354	2	2.584	2	0	1	0	1	0	1	0	3
294		min	-4621.254	3	-.087	3	0	1	0	1	0	1	-.01	2
295	15	max	3285.77	2	2.577	2	0	1	0	1	0	1	0	3
296		min	-4620.942	3	-.092	3	0	1	0	1	0	1	-.01	2
297	16	max	3286.186	2	2.57	2	0	1	0	1	0	1	0	3
298		min	-4620.63	3	-.097	3	0	1	0	1	0	1	-.011	2
299	17	max	3286.602	2	2.563	2	0	1	0	1	0	1	0	3
300		min	-4620.318	3	-.102	3	0	1	0	1	0	1	-.012	2
301	18	max	3287.018	2	2.557	2	0	1	0	1	0	1	0	3
302		min	-4620.006	3	-.108	3	0	1	0	1	0	1	-.012	2
303	19	max	3287.434	2	2.55	2	0	1	0	1	0	1	0	3
304		min	-4619.694	3	-.113	3	0	1	0	1	0	1	-.013	2
305	M7	1	max	2049.149	2	9.127	4	0	1	0	0	1	.013	2
306		min	-2266.277	3	2.143	15	0	1	0	1	0	1	0	3
307	2	max	2048.979	2	8.253	4	0	1	0	1	0	1	.01	2
308		min	-2266.405	3	1.937	15	0	1	0	1	0	1	-.002	3
309	3	max	2048.809	2	7.378	4	0	1	0	1	0	1	.007	2
310		min	-2266.532	3	1.732	15	0	1	0	1	0	1	-.004	3
311	4	max	2048.638	2	6.504	4	0	1	0	1	0	1	.004	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2266.66	3	1.526	15	0	1	0	1	0	1	-.006	3
313	5	max	2048.468	2	5.63	4	0	1	0	1	0	1	.001	2
314		min	-2266.788	3	1.321	15	0	1	0	1	0	1	-.007	3
315	6	max	2048.298	2	4.755	4	0	1	0	1	0	1	0	2
316		min	-2266.916	3	1.115	15	0	1	0	1	0	1	-.008	3
317	7	max	2048.127	2	3.881	4	0	1	0	1	0	1	-.002	15
318		min	-2267.043	3	.91	15	0	1	0	1	0	1	-.009	3
319	8	max	2047.957	2	3.006	4	0	1	0	1	0	1	-.002	15
320		min	-2267.171	3	.686	12	0	1	0	1	0	1	-.01	3
321	9	max	2047.787	2	2.228	2	0	1	0	1	0	1	-.003	15
322		min	-2267.299	3	.346	12	0	1	0	1	0	1	-.011	4
323	10	max	2047.616	2	1.546	2	0	1	0	1	0	1	-.003	15
324		min	-2267.427	3	-.04	3	0	1	0	1	0	1	-.011	4
325	11	max	2047.446	2	.865	2	0	1	0	1	0	1	-.003	15
326		min	-2267.555	3	-.551	3	0	1	0	1	0	1	-.012	4
327	12	max	2047.276	2	.183	2	0	1	0	1	0	1	-.003	15
328		min	-2267.682	3	-1.062	3	0	1	0	1	0	1	-.012	4
329	13	max	2047.105	2	-.324	15	0	1	0	1	0	1	-.003	15
330		min	-2267.81	3	-1.573	3	0	1	0	1	0	1	-.011	4
331	14	max	2046.935	2	-.529	15	0	1	0	1	0	1	-.002	15
332		min	-2267.938	3	-2.24	4	0	1	0	1	0	1	-.01	4
333	15	max	2046.764	2	-.735	15	0	1	0	1	0	1	-.002	15
334		min	-2268.066	3	-3.115	4	0	1	0	1	0	1	-.009	4
335	16	max	2046.594	2	-.94	15	0	1	0	1	0	1	-.002	15
336		min	-2268.193	3	-3.989	4	0	1	0	1	0	1	-.008	4
337	17	max	2046.424	2	-1.146	15	0	1	0	1	0	1	-.001	15
338		min	-2268.321	3	-4.864	4	0	1	0	1	0	1	-.005	4
339	18	max	2046.253	2	-1.351	15	0	1	0	1	0	1	0	15
340		min	-2268.449	3	-5.738	4	0	1	0	1	0	1	-.003	4
341	19	max	2046.083	2	-1.557	15	0	1	0	1	0	1	0	1
342		min	-2268.577	3	-6.613	4	0	1	0	1	0	1	0	1
343	M8	1	max	3150.207	2	0	1	0	1	0	1	0	1	1
344		min	-981.744	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3150.377	2	0	1	0	1	0	1	0	1	0	1
346		min	-981.616	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3150.548	2	0	1	0	1	0	1	0	1	0	1
348		min	-981.488	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3150.718	2	0	1	0	1	0	1	0	1	0	1
350		min	-981.361	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3150.888	2	0	1	0	1	0	1	0	1	0	1
352		min	-981.233	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3151.059	2	0	1	0	1	0	1	0	1	0	1
354		min	-981.105	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3151.229	2	0	1	0	1	0	1	0	1	0	1
356		min	-980.977	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3151.399	2	0	1	0	1	0	1	0	1	0	1
358		min	-980.85	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3151.57	2	0	1	0	1	0	1	0	1	0	1
360		min	-980.722	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3151.74	2	0	1	0	1	0	1	0	1	0	1
362		min	-980.594	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3151.91	2	0	1	0	1	0	1	0	1	0	1
364		min	-980.466	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3152.081	2	0	1	0	1	0	1	0	1	0	1
366		min	-980.338	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3152.251	2	0	1	0	1	0	1	0	1	0	1
368		min	-980.211	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Nov 23, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3152.421	2	0	1	0	1	0	1	0	1	0	1
370			min	-980.083	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3152.592	2	0	1	0	1	0	1	0	1	0	1
372			min	-979.955	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3152.762	2	0	1	0	1	0	1	0	1	0	1
374			min	-979.827	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3152.932	2	0	1	0	1	0	1	0	1	0	1
376			min	-979.7	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3153.103	2	0	1	0	1	0	1	0	1	0	1
378			min	-979.572	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3153.273	2	0	1	0	1	0	1	0	1	0	1
380			min	-979.444	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1122.477	2	2.159	4	-.016	15	0	1	0	2	0	1
382			min	-1529.874	3	.507	15	-.425	1	0	5	0	3	0	1
383		2	max	1122.893	2	2.15	4	-.016	15	0	1	0	10	0	15
384			min	-1529.562	3	.505	15	-.425	1	0	5	0	1	0	4
385		3	max	1123.309	2	2.141	4	-.016	15	0	1	0	10	0	15
386			min	-1529.25	3	.503	15	-.425	1	0	5	0	1	-.001	4
387		4	max	1123.725	2	2.133	4	-.016	15	0	1	0	15	0	15
388			min	-1528.938	3	.501	15	-.425	1	0	5	0	1	-.002	4
389		5	max	1124.141	2	2.124	4	-.016	15	0	1	0	15	0	15
390			min	-1528.626	3	.499	15	-.425	1	0	5	0	1	-.002	4
391		6	max	1124.556	2	2.115	4	-.016	15	0	1	0	15	0	15
392			min	-1528.314	3	.497	15	-.425	1	0	5	0	1	-.003	4
393		7	max	1124.972	2	2.107	4	-.016	15	0	1	0	15	0	15
394			min	-1528.002	3	.495	15	-.425	1	0	5	0	1	-.004	4
395		8	max	1125.388	2	2.098	4	-.016	15	0	1	0	15	0	15
396			min	-1527.69	3	.493	15	-.425	1	0	5	0	1	-.004	4
397		9	max	1125.804	2	2.089	4	-.016	15	0	1	0	15	-.001	15
398			min	-1527.378	3	.491	15	-.425	1	0	5	0	1	-.005	4
399		10	max	1126.22	2	2.08	4	-.016	15	0	1	0	15	-.001	15
400			min	-1527.066	3	.489	15	-.425	1	0	5	-.001	1	-.005	4
401		11	max	1126.636	2	2.072	4	-.016	15	0	1	0	15	-.001	15
402			min	-1526.754	3	.487	15	-.425	1	0	5	-.001	1	-.006	4
403		12	max	1127.052	2	2.063	4	-.016	15	0	1	0	15	-.002	15
404			min	-1526.443	3	.485	15	-.425	1	0	5	-.001	1	-.007	4
405		13	max	1127.468	2	2.054	4	-.016	15	0	1	0	15	-.002	15
406			min	-1526.131	3	.483	15	-.425	1	0	5	-.001	1	-.007	4
407		14	max	1127.883	2	2.046	4	-.016	15	0	1	0	15	-.002	15
408			min	-1525.819	3	.481	15	-.425	1	0	5	-.002	1	-.008	4
409		15	max	1128.299	2	2.037	4	-.016	15	0	1	0	15	-.002	15
410			min	-1525.507	3	.479	15	-.425	1	0	5	-.002	1	-.008	4
411		16	max	1128.715	2	2.028	4	-.016	15	0	1	0	15	-.002	15
412			min	-1525.195	3	.477	15	-.425	1	0	5	-.002	1	-.009	4
413		17	max	1129.131	2	2.019	4	-.016	15	0	1	0	15	-.002	15
414			min	-1524.883	3	.475	15	-.425	1	0	5	-.002	1	-.009	4
415		18	max	1129.547	2	2.011	4	-.016	15	0	1	0	15	-.002	15
416			min	-1524.571	3	.473	15	-.425	1	0	5	-.002	1	-.01	4
417		19	max	1129.963	2	2.002	4	-.016	15	0	1	0	15	-.002	15
418			min	-1524.259	3	.47	15	-.425	1	0	5	-.002	1	-.01	4
419	M11	1	max	654.331	2	9.102	4	-.004	15	0	1	0	15	.01	4
420			min	-790.451	3	2.139	15	-.11	1	0	3	0	1	.002	15
421		2	max	654.161	2	8.228	4	-.004	15	0	1	0	15	.007	2
422			min	-790.579	3	1.934	15	-.11	1	0	3	0	1	.001	12
423		3	max	653.99	2	7.353	4	-.004	15	0	1	0	15	.004	2
424			min	-790.706	3	1.728	15	-.11	1	0	3	0	1	0	3
425		4	max	653.82	2	6.479	4	-.004	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-790.834	3	1.523	15	-.11	1	0	3	0	1	-.002	3
427		5	max	653.65	2	5.604	4	-.004	15	0	1	0	15	0	15
428			min	-790.962	3	1.317	15	-.11	1	0	3	0	1	-.004	3
429		6	max	653.479	2	4.73	4	-.004	15	0	1	0	15	-.001	15
430			min	-791.09	3	1.112	15	-.11	1	0	3	0	1	-.006	4
431		7	max	653.309	2	3.855	4	-.004	15	0	1	0	15	-.002	15
432			min	-791.217	3	.906	15	-.11	1	0	3	0	1	-.008	4
433		8	max	653.139	2	2.981	4	-.004	15	0	1	0	15	-.002	15
434			min	-791.345	3	.701	15	-.11	1	0	3	0	1	-.01	4
435		9	max	652.968	2	2.106	4	-.004	15	0	1	0	15	-.003	15
436			min	-791.473	3	.495	15	-.11	1	0	3	0	1	-.011	4
437		10	max	652.798	2	1.232	4	-.004	15	0	1	0	15	-.003	15
438			min	-791.601	3	.29	15	-.11	1	0	3	0	1	-.012	4
439		11	max	652.628	2	.468	2	-.004	15	0	1	0	15	-.003	15
440			min	-791.728	3	-.052	3	-.11	1	0	3	0	1	-.012	4
441		12	max	652.457	2	-.122	15	-.004	15	0	1	0	15	-.003	15
442			min	-791.856	3	-.563	3	-.11	1	0	3	0	1	-.012	4
443		13	max	652.287	2	-.327	15	-.004	15	0	1	0	15	-.003	15
444			min	-791.984	3	-1.391	4	-.11	1	0	3	0	1	-.011	4
445		14	max	652.117	2	-.533	15	-.004	15	0	1	0	15	-.002	15
446			min	-792.112	3	-2.266	4	-.11	1	0	3	0	1	-.011	4
447		15	max	651.946	2	-.738	15	-.004	15	0	1	0	15	-.002	15
448			min	-792.239	3	-3.14	4	-.11	1	0	3	0	1	-.009	4
449		16	max	651.776	2	-.944	15	-.004	15	0	1	0	15	-.002	15
450			min	-792.367	3	-4.015	4	-.11	1	0	3	0	1	-.008	4
451		17	max	651.606	2	-1.149	15	-.004	15	0	1	0	15	-.001	15
452			min	-792.495	3	-4.889	4	-.11	1	0	3	0	1	-.005	4
453		18	max	651.435	2	-1.355	15	-.004	15	0	1	0	15	0	15
454			min	-792.623	3	-5.764	4	-.11	1	0	3	0	1	-.003	4
455		19	max	651.265	2	-1.56	15	-.004	15	0	1	0	15	0	1
456			min	-792.75	3	-6.638	4	-.11	1	0	3	0	1	0	1
457	M12	1	max	1083.946	2	0	1	6.101	1	0	1	0	15	0	1
458			min	-305.463	3	0	1	.224	15	0	1	0	1	0	1
459		2	max	1084.116	2	0	1	6.101	1	0	1	0	1	0	1
460			min	-305.335	3	0	1	.224	15	0	1	0	12	0	1
461		3	max	1084.286	2	0	1	6.101	1	0	1	0	1	0	1
462			min	-305.207	3	0	1	.224	15	0	1	0	15	0	1
463		4	max	1084.457	2	0	1	6.101	1	0	1	.002	1	0	1
464			min	-305.079	3	0	1	.224	15	0	1	0	15	0	1
465		5	max	1084.627	2	0	1	6.101	1	0	1	.002	1	0	1
466			min	-304.952	3	0	1	.224	15	0	1	0	15	0	1
467		6	max	1084.797	2	0	1	6.101	1	0	1	.003	1	0	1
468			min	-304.824	3	0	1	.224	15	0	1	0	15	0	1
469		7	max	1084.968	2	0	1	6.101	1	0	1	.004	1	0	1
470			min	-304.696	3	0	1	.224	15	0	1	0	15	0	1
471		8	max	1085.138	2	0	1	6.101	1	0	1	.004	1	0	1
472			min	-304.568	3	0	1	.224	15	0	1	0	15	0	1
473		9	max	1085.308	2	0	1	6.101	1	0	1	.005	1	0	1
474			min	-304.44	3	0	1	.224	15	0	1	0	15	0	1
475		10	max	1085.479	2	0	1	6.101	1	0	1	.006	1	0	1
476			min	-304.313	3	0	1	.224	15	0	1	0	15	0	1
477		11	max	1085.649	2	0	1	6.101	1	0	1	.006	1	0	1
478			min	-304.185	3	0	1	.224	15	0	1	0	15	0	1
479		12	max	1085.819	2	0	1	6.101	1	0	1	.007	1	0	1
480			min	-304.057	3	0	1	.224	15	0	1	0	15	0	1
481		13	max	1085.99	2	0	1	6.101	1	0	1	.008	1	0	1
482			min	-303.929	3	0	1	.224	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1086.16	2	0	1	6.101	1	0	1	.009	1	0	1
484			min	-303.802	3	0	1	.224	15	0	1	0	15	0	1
485		15	max	1086.33	2	0	1	6.101	1	0	1	.009	1	0	1
486			min	-303.674	3	0	1	.224	15	0	1	0	15	0	1
487		16	max	1086.501	2	0	1	6.101	1	0	1	.01	1	0	1
488			min	-303.546	3	0	1	.224	15	0	1	0	15	0	1
489		17	max	1086.671	2	0	1	6.101	1	0	1	.011	1	0	1
490			min	-303.418	3	0	1	.224	15	0	1	0	15	0	1
491		18	max	1086.842	2	0	1	6.101	1	0	1	.011	1	0	1
492			min	-303.291	3	0	1	.224	15	0	1	0	15	0	1
493		19	max	1087.012	2	0	1	6.101	1	0	1	.012	1	0	1
494			min	-303.163	3	0	1	.224	15	0	1	0	15	0	1
495	M1	1	max	118.912	1	811.584	3	-1.353	15	0	1	.101	1	0	15
496			min	4.288	15	-445.892	2	-36.331	1	0	3	.004	15	-.011	2
497		2	max	119.488	1	810.397	3	-1.353	15	0	1	.078	1	.266	2
498			min	4.462	15	-447.475	2	-36.331	1	0	3	.003	15	-.514	3
499		3	max	511.731	3	606.184	2	-1.337	15	0	3	.056	1	.532	2
500			min	-319.296	2	-642.603	3	-36.001	1	0	2	.002	15	-1	3
501		4	max	512.163	3	604.601	2	-1.337	15	0	3	.033	1	.17	1
502			min	-318.72	2	-643.79	3	-36.001	1	0	2	.001	15	-.601	3
503		5	max	512.595	3	603.018	2	-1.337	15	0	3	.011	1	-.006	15
504			min	-318.144	2	-644.977	3	-36.001	1	0	2	0	15	-.218	2
505		6	max	513.027	3	601.435	2	-1.337	15	0	3	0	15	.199	3
506			min	-317.567	2	-646.165	3	-36.001	1	0	2	-.011	1	-.592	2
507		7	max	513.459	3	599.851	2	-1.337	15	0	3	-.001	15	.601	3
508			min	-316.991	2	-647.352	3	-36.001	1	0	2	-.034	1	-.965	2
509		8	max	513.891	3	598.268	2	-1.337	15	0	3	-.002	15	1.003	3
510			min	-316.415	2	-648.54	3	-36.001	1	0	2	-.056	1	-1.336	2
511		9	max	525.535	3	50.317	2	-2.301	15	0	9	.038	1	1.166	3
512			min	-269.252	2	.481	15	-62.138	1	0	3	.001	15	-1.521	2
513		10	max	525.967	3	48.733	2	-2.301	15	0	9	0	10	1.143	3
514			min	-268.676	2	.004	15	-62.138	1	0	3	0	1	-1.552	2
515		11	max	526.399	3	47.15	2	-2.301	15	0	9	-.001	15	1.121	3
516			min	-268.099	2	-1.958	4	-62.138	1	0	3	-.039	1	-1.582	2
517		12	max	537.671	3	438.416	3	-1.29	15	0	2	.055	1	.986	3
518			min	-220.757	2	-706.733	2	-35.045	1	0	3	.002	15	-1.405	2
519		13	max	538.103	3	437.229	3	-1.29	15	0	2	.033	1	.714	3
520			min	-220.181	2	-708.317	2	-35.045	1	0	3	.001	15	-.966	2
521		14	max	538.535	3	436.042	3	-1.29	15	0	2	.012	1	.443	3
522			min	-219.605	2	-709.9	2	-35.045	1	0	3	0	15	-.526	2
523		15	max	538.967	3	434.854	3	-1.29	15	0	2	0	15	.173	3
524			min	-219.028	2	-711.483	2	-35.045	1	0	3	-.01	1	-1.106	1
525		16	max	539.399	3	433.667	3	-1.29	15	0	2	-.001	15	.357	2
526			min	-218.452	2	-713.066	2	-35.045	1	0	3	-.032	1	-.096	3
527		17	max	539.832	3	432.479	3	-1.29	15	0	2	-.002	15	.8	2
528			min	-217.876	2	-714.649	2	-35.045	1	0	3	-.054	1	-.365	3
529		18	max	-4.475	15	659.524	2	-1.51	15	0	3	-.003	15	.405	2
530			min	-120.154	1	-309.708	3	-40.866	1	0	2	-.078	1	-.181	3
531		19	max	-4.301	15	657.94	2	-1.51	15	0	3	-.004	15	.011	3
532			min	-119.577	1	-310.896	3	-40.866	1	0	2	-.103	1	-.005	1
533	M5	1	max	274.922	1	2645.998	3	0	1	0	1	0	1	.023	2
534			min	4.618	12	-1559.682	2	0	1	0	1	0	1	0	15
535		2	max	275.498	1	2644.811	3	0	1	0	1	0	1	.991	2
536			min	4.906	12	-1561.265	2	0	1	0	1	0	1	-1.621	3
537		3	max	1515.181	3	1502.936	2	0	1	0	1	0	1	1.927	2
538			min	-956.116	2	-1765.25	3	0	1	0	1	0	1	-3.214	3
539		4	max	1515.613	3	1501.353	2	0	1	0	1	0	1	.995	2





Company : Schletter, Inc.  
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 Job Number :  
 Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-955.54	2	-1766.438	3	0	1	0	1	0	1	-2.118	3
541		5	max	1516.045	3	1499.77	2	0	1	0	1	0	1	.114	1
542			min	-954.964	2	-1767.625	3	0	1	0	1	0	1	-1.022	3
543		6	max	1516.477	3	1498.187	2	0	1	0	1	0	1	.076	3
544			min	-954.387	2	-1768.812	3	0	1	0	1	0	1	-.867	2
545		7	max	1516.909	3	1496.604	2	0	1	0	1	0	1	1.174	3
546			min	-953.811	2	-1770	3	0	1	0	1	0	1	-1.796	2
547		8	max	1517.342	3	1495.02	2	0	1	0	1	0	1	2.273	3
548			min	-953.235	2	-1771.187	3	0	1	0	1	0	1	-2.724	2
549		9	max	1523.195	3	173.048	2	0	1	0	1	0	1	2.624	3
550			min	-843.976	2	.476	15	0	1	0	1	0	1	-3.126	2
551		10	max	1523.627	3	171.465	2	0	1	0	1	0	1	2.528	3
552			min	-843.4	2	-.002	15	0	1	0	1	0	1	-3.233	2
553		11	max	1524.059	3	169.882	2	0	1	0	1	0	1	2.433	3
554			min	-842.823	2	-1.9	4	0	1	0	1	0	1	-3.339	2
555		12	max	1530.657	3	1130.63	3	0	1	0	1	0	1	2.121	3
556			min	-733.923	2	-1837.229	2	0	1	0	1	0	1	-2.981	2
557		13	max	1531.089	3	1129.443	3	0	1	0	1	0	1	1.42	3
558			min	-733.347	2	-1838.812	2	0	1	0	1	0	1	-1.84	2
559		14	max	1531.521	3	1128.255	3	0	1	0	1	0	1	.719	3
560			min	-732.771	2	-1840.395	2	0	1	0	1	0	1	-.698	2
561		15	max	1531.954	3	1127.068	3	0	1	0	1	0	1	.444	2
562			min	-732.195	2	-1841.978	2	0	1	0	1	0	1	.001	15
563		16	max	1532.386	3	1125.881	3	0	1	0	1	0	1	1.588	2
564			min	-731.618	2	-1843.562	2	0	1	0	1	0	1	-.68	3
565		17	max	1532.818	3	1124.693	3	0	1	0	1	0	1	2.733	2
566			min	-731.042	2	-1845.145	2	0	1	0	1	0	1	-1.378	3
567		18	max	-6.363	12	2231.605	2	0	1	0	1	0	1	1.392	2
568			min	-274.167	1	-1111.721	3	0	1	0	1	0	1	-.713	3
569		19	max	-6.075	12	2230.021	2	0	1	0	1	0	1	.01	1
570			min	-273.591	1	-1112.909	3	0	1	0	1	0	1	-.023	3
571	M9	1	max	118.912	1	811.584	3	36.331	1	0	3	-.004	15	0	15
572			min	4.288	15	-445.892	2	1.353	15	0	1	-.101	1	-.011	2
573		2	max	119.488	1	810.397	3	36.331	1	0	3	-.003	15	.266	2
574			min	4.462	15	-447.475	2	1.353	15	0	1	-.078	1	-.514	3
575		3	max	511.731	3	606.184	2	36.001	1	0	2	-.002	15	.532	2
576			min	-319.296	2	-642.603	3	1.337	15	0	3	-.056	1	-1	3
577		4	max	512.163	3	604.601	2	36.001	1	0	2	-.001	15	.17	1
578			min	-318.72	2	-643.79	3	1.337	15	0	3	-.033	1	-.601	3
579		5	max	512.595	3	603.018	2	36.001	1	0	2	0	15	-.006	15
580			min	-318.144	2	-644.977	3	1.337	15	0	3	-.011	1	-.218	2
581		6	max	513.027	3	601.435	2	36.001	1	0	2	.011	1	.199	3
582			min	-317.567	2	-646.165	3	1.337	15	0	3	0	15	-.592	2
583		7	max	513.459	3	599.851	2	36.001	1	0	2	.034	1	.601	3
584			min	-316.991	2	-647.352	3	1.337	15	0	3	.001	15	-.965	2
585		8	max	513.891	3	598.268	2	36.001	1	0	2	.056	1	1.003	3
586			min	-316.415	2	-648.54	3	1.337	15	0	3	.002	15	-1.336	2
587		9	max	525.535	3	50.317	2	62.138	1	0	3	-.001	15	1.166	3
588			min	-269.252	2	.481	15	2.301	15	0	9	-.038	1	-1.521	2
589		10	max	525.967	3	48.733	2	62.138	1	0	3	0	1	1.143	3
590			min	-268.676	2	.004	15	2.301	15	0	9	0	10	-1.552	2
591		11	max	526.399	3	47.15	2	62.138	1	0	3	.039	1	1.121	3
592			min	-268.099	2	-1.958	4	2.301	15	0	9	.001	15	-1.582	2
593		12	max	537.671	3	438.416	3	35.045	1	0	3	-.002	15	.986	3
594			min	-220.757	2	-706.733	2	1.29	15	0	2	-.055	1	-1.405	2
595		13	max	538.103	3	437.229	3	35.045	1	0	3	-.001	15	.714	3
596			min	-220.181	2	-708.317	2	1.29	15	0	2	-.033	1	-.966	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	538.535	3	436.042	3	35.045	1	0	3	0	15	.443	3
598		min	-219.605	2	-709.9	2	1.29	15	0	2	-.012	1	-.526	2
599	15	max	538.967	3	434.854	3	35.045	1	0	3	.01	1	.173	3
600		min	-219.028	2	-711.483	2	1.29	15	0	2	0	15	-.106	1
601	16	max	539.399	3	433.667	3	35.045	1	0	3	.032	1	.357	2
602		min	-218.452	2	-713.066	2	1.29	15	0	2	.001	15	-.096	3
603	17	max	539.832	3	432.479	3	35.045	1	0	3	.054	1	.8	2
604		min	-217.876	2	-714.649	2	1.29	15	0	2	.002	15	-.365	3
605	18	max	-4.475	15	659.524	2	40.866	1	0	2	.078	1	.405	2
606		min	-120.154	1	-309.708	3	1.51	15	0	3	.003	15	-.181	3
607	19	max	-4.301	15	657.94	2	40.866	1	0	2	.103	1	.011	3
608		min	-119.577	1	-310.896	3	1.51	15	0	3	.004	15	-.005	1

### Envelope Member Section Deflections

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC		
1	M13	1	max	0	1	.245	2	.01	3	1.666e-2	2	NC	1	NC	1	
2			min	0	15	-.08	3	-.006	2	-5.145e-3	3	NC	1	NC	1	
3		2	max	0	1	.202	2	.012	3	1.739e-2	2	NC	4	NC	1	
4			min	0	15	.005	15	-.004	10	-4.492e-3	3	1333.541	3	NC	1	
5		3	max	0	1	.171	2	.019	1	1.812e-2	2	NC	4	NC	2	
6			min	0	15	.004	15	-.003	10	-3.839e-3	3	729.531	3	7762.792	1	
7		4	max	0	1	.197	3	.028	1	1.885e-2	2	NC	5	NC	2	
8			min	0	15	.003	15	-.004	10	-3.187e-3	3	563.31	3	5391.136	1	
9		5	max	0	1	.22	3	.031	1	1.958e-2	2	NC	4	NC	2	
10			min	0	15	.003	15	-.004	10	-2.534e-3	3	520.452	3	4787.939	1	
11		6	max	0	1	.203	3	.029	1	2.031e-2	2	NC	4	NC	2	
12			min	0	15	.004	15	-.006	10	-1.881e-3	3	551.62	3	5221.136	1	
13		7	max	0	1	.236	2	.026	3	2.104e-2	2	NC	2	NC	2	
14			min	0	15	.005	15	-.008	10	-1.229e-3	3	667.324	3	7295.26	1	
15		8	max	0	1	.286	2	.027	3	2.177e-2	2	NC	4	NC	1	
16			min	0	15	.006	15	-.012	2	-5.759e-4	3	932.06	3	8968.379	3	
17		9	max	0	1	.329	2	.028	3	2.25e-2	2	NC	4	NC	1	
18			min	0	15	.007	15	-.017	2	7.679e-5	3	1479.622	3	8511.212	3	
19		10	max	0	1	.349	2	.029	3	2.322e-2	2	NC	4	NC	1	
20			min	0	1	-.003	3	-.02	2	4.503e-4	15	1511.315	2	8379.246	3	
21		11	max	0	15	.329	2	.028	3	2.25e-2	2	NC	4	NC	1	
22			min	0	1	.007	15	-.017	2	7.679e-5	3	1479.622	3	8511.212	3	
23		12	max	0	15	.286	2	.027	3	2.177e-2	2	NC	4	NC	1	
24			min	0	1	.006	15	-.012	2	-5.759e-4	3	932.06	3	8968.379	3	
25		13	max	0	15	.236	2	.026	3	2.104e-2	2	NC	2	NC	2	
26			min	0	1	.005	15	-.008	10	-1.229e-3	3	667.324	3	7295.26	1	
27		14	max	0	15	.203	3	.029	1	2.031e-2	2	NC	4	NC	2	
28			min	0	1	.004	15	-.006	10	-1.881e-3	3	551.62	3	5221.136	1	
29		15	max	0	15	.22	3	.031	1	1.958e-2	2	NC	4	NC	2	
30			min	0	1	.003	15	-.004	10	-2.534e-3	3	520.452	3	4787.939	1	
31		16	max	0	15	.197	3	.028	1	1.885e-2	2	NC	5	NC	2	
32			min	0	1	.003	15	-.004	10	-3.187e-3	3	563.31	3	5391.136	1	
33		17	max	0	15	.171	2	.019	1	1.812e-2	2	NC	4	NC	2	
34			min	0	1	.004	15	-.003	10	-3.839e-3	3	729.531	3	7762.792	1	
35		18	max	0	15	.202	2	.012	3	1.739e-2	2	NC	4	NC	1	
36			min	0	1	.005	15	-.004	10	-4.492e-3	3	1333.541	3	NC	1	
37		19	max	0	15	.245	2	.01	3	1.666e-2	2	NC	1	NC	1	
38			min	0	1	-.08	3	-.006	2	-5.145e-3	3	NC	1	NC	1	
39		M14	1	max	0	1	.491	3	.009	3	9.044e-3	2	NC	1	NC	1
40				min	0	15	-.707	2	-.006	2	-7.346e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.655	3	.01	3	1.013e-2	2	NC	5	NC	1
42			min	0	15	-.877	2	-.004	2	-8.346e-3	3	917.661	2	NC	1
43		3	max	0	1	.803	3	.014	1	1.122e-2	2	NC	5	NC	1
44			min	0	15	-1.033	2	-.003	10	-9.346e-3	3	478.713	2	NC	1
45		4	max	0	1	.921	3	.022	1	1.23e-2	2	NC	5	NC	2
46			min	0	15	-1.164	2	-.003	10	-1.035e-2	3	341.172	2	6751.669	1
47		5	max	0	1	1.002	3	.026	1	1.339e-2	2	NC	15	NC	2
48			min	0	15	-1.265	2	-.004	10	-1.135e-2	3	279.663	2	5704.298	1
49		6	max	0	1	1.046	3	.025	1	1.448e-2	2	NC	15	NC	2
50			min	0	15	-1.332	2	-.005	10	-1.235e-2	3	249.46	2	6017.436	1
51		7	max	0	1	1.056	3	.023	3	1.556e-2	2	NC	15	NC	2
52			min	0	15	-1.369	2	-.007	10	-1.335e-2	3	235.752	2	8192.822	1
53		8	max	0	1	1.042	3	.024	3	1.665e-2	2	NC	15	NC	1
54			min	0	15	-1.38	2	-.011	2	-1.435e-2	3	231.858	2	NC	1
55		9	max	0	1	1.018	3	.025	3	1.774e-2	2	NC	15	NC	1
56			min	0	15	-1.376	2	-.016	2	-1.535e-2	3	233.285	2	9636.541	3
57		10	max	0	1	1.004	3	.025	3	1.882e-2	2	NC	15	NC	1
58			min	0	1	-1.371	2	-.018	2	-1.635e-2	3	235.134	2	9459.376	3
59		11	max	0	15	1.018	3	.025	3	1.774e-2	2	NC	15	NC	1
60			min	0	1	-1.376	2	-.016	2	-1.535e-2	3	233.285	2	9636.541	3
61		12	max	0	15	1.042	3	.024	3	1.665e-2	2	NC	15	NC	1
62			min	0	1	-1.38	2	-.011	2	-1.435e-2	3	231.858	2	NC	1
63		13	max	0	15	1.056	3	.023	3	1.556e-2	2	NC	15	NC	2
64			min	0	1	-1.369	2	-.007	10	-1.335e-2	3	235.752	2	8192.822	1
65		14	max	0	15	1.046	3	.025	1	1.448e-2	2	NC	15	NC	2
66			min	0	1	-1.332	2	-.005	10	-1.235e-2	3	249.46	2	6017.436	1
67		15	max	0	15	1.002	3	.026	1	1.339e-2	2	NC	15	NC	2
68			min	0	1	-1.265	2	-.004	10	-1.135e-2	3	279.663	2	5704.298	1
69		16	max	0	15	.921	3	.022	1	1.23e-2	2	NC	5	NC	2
70			min	0	1	-1.164	2	-.003	10	-1.035e-2	3	341.172	2	6751.669	1
71		17	max	0	15	.803	3	.014	1	1.122e-2	2	NC	5	NC	1
72			min	0	1	-1.033	2	-.003	10	-9.346e-3	3	478.713	2	NC	1
73		18	max	0	15	.655	3	.01	3	1.013e-2	2	NC	5	NC	1
74			min	0	1	-.877	2	-.004	2	-8.346e-3	3	917.661	2	NC	1
75		19	max	0	15	.491	3	.009	3	9.044e-3	2	NC	1	NC	1
76			min	0	1	-.707	2	-.006	2	-7.346e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.502	3	.008	3	6.224e-3	3	NC	1	NC	1
78			min	0	1	-.706	2	-.005	2	-9.365e-3	2	NC	1	NC	1
79		2	max	0	15	.632	3	.009	3	7.05e-3	3	NC	5	NC	1
80			min	0	1	-.903	2	-.003	2	-1.05e-2	2	792.803	2	NC	1
81		3	max	0	15	.751	3	.014	1	7.876e-3	3	NC	5	NC	1
82			min	0	1	-1.08	2	-.003	10	-1.163e-2	2	416.698	2	NC	1
83		4	max	0	15	.853	3	.022	1	8.702e-3	3	NC	5	NC	2
84			min	0	1	-1.225	2	-.003	10	-1.276e-2	2	300.561	2	6694.528	1
85		5	max	0	15	.932	3	.027	1	9.527e-3	3	NC	5	NC	2
86			min	0	1	-1.329	2	-.004	10	-1.389e-2	2	250.462	2	5649.702	1
87		6	max	0	15	.987	3	.025	1	1.035e-2	3	NC	15	NC	2
88			min	0	1	-1.39	2	-.005	10	-1.502e-2	2	228.13	2	5942.589	1
89		7	max	0	15	1.019	3	.021	3	1.118e-2	3	NC	15	NC	2
90			min	0	1	-1.412	2	-.006	10	-1.615e-2	2	221.02	2	8032.241	1
91		8	max	0	15	1.031	3	.022	3	1.201e-2	3	NC	15	NC	1
92			min	0	1	-1.404	2	-.01	2	-1.728e-2	2	223.304	2	NC	1
93		9	max	0	15	1.031	3	.023	3	1.283e-2	3	NC	15	NC	1
94			min	0	1	-1.384	2	-.014	2	-1.841e-2	2	230.156	2	NC	1
95		10	max	0	1	1.028	3	.024	3	1.366e-2	3	NC	15	NC	1
96			min	0	1	-1.371	2	-.017	2	-1.954e-2	2	234.605	2	NC	1
97		11	max	0	1	1.031	3	.023	3	1.283e-2	3	NC	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.384	2	-.014	2	-1.841e-2	2	230.156	2	NC	1
99		max	0	1	1.031	3	.022	3	1.201e-2	3	NC	15	NC	1
100		min	0	15	-1.404	2	-.01	2	-1.728e-2	2	223.304	2	NC	1
101		max	0	1	1.019	3	.021	3	1.118e-2	3	NC	15	NC	2
102		min	0	15	-1.412	2	-.006	10	-1.615e-2	2	221.02	2	8032.241	1
103		max	0	1	.987	3	.025	1	1.035e-2	3	NC	15	NC	2
104		min	0	15	-1.39	2	-.005	10	-1.502e-2	2	228.13	2	5942.589	1
105		max	0	1	.932	3	.027	1	9.527e-3	3	NC	5	NC	2
106		min	0	15	-1.329	2	-.004	10	-1.389e-2	2	250.462	2	5649.702	1
107		max	0	1	.853	3	.022	1	8.702e-3	3	NC	5	NC	2
108		min	0	15	-1.225	2	-.003	10	-1.276e-2	2	300.561	2	6694.528	1
109		max	0	1	.751	3	.014	1	7.876e-3	3	NC	5	NC	1
110		min	0	15	-1.08	2	-.003	10	-1.163e-2	2	416.698	2	NC	1
111		max	0	1	.632	3	.009	3	7.05e-3	3	NC	5	NC	1
112		min	0	15	-.903	2	-.003	2	-1.05e-2	2	792.803	2	NC	1
113		max	0	1	.502	3	.008	3	6.224e-3	3	NC	1	NC	1
114		min	0	15	-.706	2	-.005	2	-9.365e-3	2	NC	1	NC	1
115	M16	max	0	15	.219	2	.007	3	1.216e-2	3	NC	1	NC	1
116		min	0	1	-.182	3	-.005	2	-1.423e-2	2	NC	1	NC	1
117		max	0	15	.142	1	.008	3	1.288e-2	3	NC	4	NC	1
118		min	0	1	-.157	3	-.003	10	-1.446e-2	2	2012.701	2	NC	1
119		max	0	15	.095	1	.019	1	1.359e-2	3	NC	4	NC	2
120		min	0	1	-.14	3	-.002	10	-1.47e-2	2	1123.924	2	7709.56	1
121		max	0	15	.07	1	.028	1	1.431e-2	3	NC	4	NC	2
122		min	0	1	-.136	3	-.002	10	-1.493e-2	2	901.467	2	5318.366	1
123		max	0	15	.07	1	.032	1	1.502e-2	3	NC	4	NC	2
124		min	0	1	-.146	3	-.003	10	-1.516e-2	2	889.786	2	4683.792	1
125		max	0	15	.096	1	.03	1	1.574e-2	3	NC	3	NC	2
126		min	0	1	-.17	3	-.004	10	-1.539e-2	2	1064.431	2	5038.338	1
127		max	0	15	.142	1	.022	1	1.645e-2	3	NC	4	NC	2
128		min	0	1	-.204	3	-.005	10	-1.562e-2	2	1667.034	2	6836.714	1
129		max	0	15	.196	1	.02	3	1.717e-2	3	NC	1	NC	1
130		min	0	1	-.242	3	-.008	2	-1.586e-2	2	2585.621	3	NC	1
131		max	0	15	.247	2	.02	3	1.788e-2	3	NC	4	NC	1
132		min	0	1	-.274	3	-.013	2	-1.609e-2	2	1686.338	3	NC	1
133		max	0	1	.273	2	.02	3	1.86e-2	3	NC	4	NC	1
134		min	0	1	-.288	3	-.015	2	-1.632e-2	2	1463.436	3	NC	1
135		max	0	1	.247	2	.02	3	1.788e-2	3	NC	4	NC	1
136		min	0	15	-.274	3	-.013	2	-1.609e-2	2	1686.338	3	NC	1
137		max	0	1	.196	1	.02	3	1.717e-2	3	NC	1	NC	1
138		min	0	15	-.242	3	-.008	2	-1.586e-2	2	2585.621	3	NC	1
139		max	0	1	.142	1	.022	1	1.645e-2	3	NC	4	NC	2
140		min	0	15	-.204	3	-.005	10	-1.562e-2	2	1667.034	2	6836.714	1
141		max	0	1	.096	1	.03	1	1.574e-2	3	NC	3	NC	2
142		min	0	15	-.17	3	-.004	10	-1.539e-2	2	1064.431	2	5038.338	1
143		max	0	1	.07	1	.032	1	1.502e-2	3	NC	4	NC	2
144		min	0	15	-.146	3	-.003	10	-1.516e-2	2	889.786	2	4683.792	1
145		max	0	1	.07	1	.028	1	1.431e-2	3	NC	4	NC	2
146		min	0	15	-.136	3	-.002	10	-1.493e-2	2	901.467	2	5318.366	1
147		max	0	1	.095	1	.019	1	1.359e-2	3	NC	4	NC	2
148		min	0	15	-.14	3	-.002	10	-1.47e-2	2	1123.924	2	7709.56	1
149		max	0	1	.142	1	.008	3	1.288e-2	3	NC	4	NC	1
150		min	0	15	-.157	3	-.003	10	-1.446e-2	2	2012.701	2	NC	1
151		max	0	1	.219	2	.007	3	1.216e-2	3	NC	1	NC	1
152		min	0	15	-.182	3	-.005	2	-1.423e-2	2	NC	1	NC	1
153	M2	max	.007	2	.009	2	.005	1	-3.698e-6	15	NC	1	NC	1
154		min	-.009	3	-.014	3	0	15	-9.914e-5	1	6857.386	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	2	.008	2	.004	1	-3.467e-6	15	NC	1	NC	1
156		min	-.008	3	-.013	3	0	15	-9.293e-5	1	7820.882	2	NC	1
157	3	max	.006	2	.007	2	.004	1	-3.237e-6	15	NC	1	NC	1
158		min	-.008	3	-.013	3	0	15	-8.672e-5	1	9084.039	2	NC	1
159	4	max	.005	2	.006	2	.004	1	-3.006e-6	15	NC	1	NC	1
160		min	-.007	3	-.012	3	0	15	-8.051e-5	1	NC	1	NC	1
161	5	max	.005	2	.005	2	.003	1	-2.775e-6	15	NC	1	NC	1
162		min	-.007	3	-.012	3	0	15	-7.43e-5	1	NC	1	NC	1
163	6	max	.005	2	.004	2	.003	1	-2.545e-6	15	NC	1	NC	1
164		min	-.006	3	-.011	3	0	15	-6.809e-5	1	NC	1	NC	1
165	7	max	.004	2	.003	2	.002	1	-2.314e-6	15	NC	1	NC	1
166		min	-.006	3	-.01	3	0	15	-6.188e-5	1	NC	1	NC	1
167	8	max	.004	2	.002	2	.002	1	-2.083e-6	15	NC	1	NC	1
168		min	-.005	3	-.01	3	0	15	-5.567e-5	1	NC	1	NC	1
169	9	max	.004	2	.001	2	.002	1	-1.841e-6	10	NC	1	NC	1
170		min	-.005	3	-.009	3	0	15	-4.946e-5	1	NC	1	NC	1
171	10	max	.003	2	0	2	.001	1	-1.56e-6	10	NC	1	NC	1
172		min	-.004	3	-.008	3	0	15	-4.325e-5	1	NC	1	NC	1
173	11	max	.003	2	0	2	.001	1	-1.28e-6	10	NC	1	NC	1
174		min	-.004	3	-.008	3	0	15	-3.704e-5	1	NC	1	NC	1
175	12	max	.003	2	0	2	0	1	-9.996e-7	10	NC	1	NC	1
176		min	-.003	3	-.007	3	0	15	-3.083e-5	1	NC	1	NC	1
177	13	max	.002	2	-.001	15	0	1	-7.193e-7	10	NC	1	NC	1
178		min	-.003	3	-.006	3	0	15	-2.462e-5	1	NC	1	NC	1
179	14	max	.002	2	0	15	0	1	-4.389e-7	10	NC	1	NC	1
180		min	-.002	3	-.005	3	0	15	-1.842e-5	1	NC	1	NC	1
181	15	max	.001	2	0	15	0	1	-1.586e-7	10	NC	1	NC	1
182		min	-.002	3	-.004	3	0	15	-1.221e-5	1	NC	1	NC	1
183	16	max	.001	2	0	15	0	1	1.218e-7	10	NC	1	NC	1
184		min	-.001	3	-.003	3	0	15	-5.996e-6	1	NC	1	NC	1
185	17	max	0	2	0	15	0	1	6.284e-7	2	NC	1	NC	1
186		min	0	3	-.002	3	0	15	-1.293e-6	3	NC	1	NC	1
187	18	max	0	2	0	15	0	1	6.423e-6	1	NC	1	NC	1
188		min	0	3	-.001	3	0	15	-1.196e-7	3	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.263e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	4.534e-7	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	-1.442e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-3.985e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	9.556e-6	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	3.521e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	2.31e-5	1	NC	1	NC	1
196		min	0	2	-.005	4	0	15	8.484e-7	15	NC	1	NC	1
197	4	max	.001	3	-.002	15	0	1	3.664e-5	1	NC	1	NC	1
198		min	-.001	2	-.008	4	0	15	1.345e-6	15	NC	1	NC	1
199	5	max	.002	3	-.003	15	0	1	5.018e-5	1	NC	1	NC	1
200		min	-.001	2	-.011	4	0	15	1.841e-6	15	9263.067	4	NC	1
201	6	max	.002	3	-.003	15	0	1	6.372e-5	1	NC	1	NC	1
202		min	-.002	2	-.014	4	0	15	2.337e-6	15	7435.447	4	NC	1
203	7	max	.003	3	-.004	15	0	1	7.726e-5	1	NC	5	NC	1
204		min	-.002	2	-.016	4	0	15	2.834e-6	15	6338.796	4	NC	1
205	8	max	.003	3	-.004	15	0	1	9.08e-5	1	NC	5	NC	1
206		min	-.002	2	-.018	4	0	15	3.33e-6	15	5661.831	4	NC	1
207	9	max	.003	3	-.005	15	0	1	1.043e-4	1	NC	5	NC	1
208		min	-.003	2	-.02	4	0	15	3.826e-6	15	5258.368	4	NC	1
209	10	max	.004	3	-.005	15	.001	1	1.179e-4	1	NC	5	NC	1
210		min	-.003	2	-.021	4	0	15	4.322e-6	15	5057.149	4	NC	1
211	11	max	.004	3	-.005	15	.001	1	1.314e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.021	4	0	15	4.819e-6	15	5028.174	4	NC	1
213		max	.005	3	-.005	15	.002	1	1.45e-4	1	NC	5	NC	1
214		min	-.004	2	-.02	4	0	15	5.315e-6	15	5171.142	4	NC	1
215		max	.005	3	-.004	15	.002	1	1.585e-4	1	NC	5	NC	1
216		min	-.004	2	-.019	4	0	15	5.811e-6	15	5516.767	4	NC	1
217		max	.006	3	-.004	15	.002	1	1.72e-4	1	NC	5	NC	1
218		min	-.005	2	-.017	4	0	15	6.308e-6	15	6143.024	4	NC	1
219		max	.006	3	-.003	15	.003	1	1.856e-4	1	NC	2	NC	1
220		min	-.005	2	-.015	4	0	15	6.804e-6	15	7223.997	4	NC	1
221		max	.006	3	-.003	15	.003	1	1.991e-4	1	NC	1	NC	1
222		min	-.005	2	-.012	4	0	15	7.3e-6	15	9182.26	4	NC	1
223		max	.007	3	-.002	15	.003	1	2.127e-4	1	NC	1	NC	1
224		min	-.006	2	-.008	4	0	15	7.796e-6	15	NC	1	NC	1
225		max	.007	3	-.001	15	.004	1	2.262e-4	1	NC	1	NC	1
226		min	-.006	2	-.005	1	0	15	8.293e-6	15	NC	1	NC	1
227		max	.008	3	0	15	.004	1	2.397e-4	1	NC	1	NC	1
228		min	-.006	2	-.002	1	0	15	8.789e-6	15	NC	1	NC	1
229	M4	max	.003	2	.006	2	0	15	4.203e-5	1	NC	1	NC	2
230		min	0	3	-.008	3	-.004	1	1.562e-6	15	NC	1	5577.399	1
231		max	.002	2	.006	2	0	15	4.203e-5	1	NC	1	NC	2
232		min	0	3	-.007	3	-.004	1	1.562e-6	15	NC	1	6065.291	1
233		max	.002	2	.005	2	0	15	4.203e-5	1	NC	1	NC	2
234		min	0	3	-.007	3	-.004	1	1.562e-6	15	NC	1	6645.943	1
235		max	.002	2	.005	2	0	15	4.203e-5	1	NC	1	NC	2
236		min	0	3	-.007	3	-.003	1	1.562e-6	15	NC	1	7343.454	1
237		max	.002	2	.005	2	0	15	4.203e-5	1	NC	1	NC	2
238		min	0	3	-.006	3	-.003	1	1.562e-6	15	NC	1	8190.573	1
239		max	.002	2	.004	2	0	15	4.203e-5	1	NC	1	NC	2
240		min	0	3	-.006	3	-.003	1	1.562e-6	15	NC	1	9232.72	1
241		max	.002	2	.004	2	0	15	4.203e-5	1	NC	1	NC	1
242		min	0	3	-.005	3	-.002	1	1.562e-6	15	NC	1	NC	1
243		max	.002	2	.004	2	0	15	4.203e-5	1	NC	1	NC	1
244		min	0	3	-.005	3	-.002	1	1.562e-6	15	NC	1	NC	1
245		max	.001	2	.003	2	0	15	4.203e-5	1	NC	1	NC	1
246		min	0	3	-.004	3	-.002	1	1.562e-6	15	NC	1	NC	1
247		max	.001	2	.003	2	0	15	4.203e-5	1	NC	1	NC	1
248		min	0	3	-.004	3	-.001	1	1.562e-6	15	NC	1	NC	1
249		max	.001	2	.003	2	0	15	4.203e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.001	1	1.562e-6	15	NC	1	NC	1
251		max	.001	2	.002	2	0	15	4.203e-5	1	NC	1	NC	1
252		min	0	3	-.003	3	0	1	1.562e-6	15	NC	1	NC	1
253		max	0	2	.002	2	0	15	4.203e-5	1	NC	1	NC	1
254		min	0	3	-.003	3	0	1	1.562e-6	15	NC	1	NC	1
255		max	0	2	.002	2	0	15	4.203e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	1.562e-6	15	NC	1	NC	1
257		max	0	2	.001	2	0	15	4.203e-5	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	1.562e-6	15	NC	1	NC	1
259		max	0	2	0	2	0	15	4.203e-5	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	1.562e-6	15	NC	1	NC	1
261		max	0	2	0	2	0	15	4.203e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	1.562e-6	15	NC	1	NC	1
263		max	0	2	0	2	0	15	4.203e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	1.562e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	4.203e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	1.562e-6	15	NC	1	NC	1
267	M6	max	.019	2	.028	2	0	1	0	1	NC	4	NC	1
268		min	-.027	3	-.04	3	0	1	0	1	1529.055	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.018	2	.025	2	0	1	0	1	NC	4	NC	1
270		min	-.025	3	-.037	3	0	1	0	1	1619.576	3	NC	1
271	3	max	.017	2	.023	2	0	1	0	1	NC	4	NC	1
272		min	-.024	3	-.035	3	0	1	0	1	1721.482	3	NC	1
273	4	max	.016	2	.021	2	0	1	0	1	NC	4	NC	1
274		min	-.022	3	-.033	3	0	1	0	1	1837.057	3	NC	1
275	5	max	.015	2	.018	2	0	1	0	1	NC	4	NC	1
276		min	-.021	3	-.031	3	0	1	0	1	1969.237	3	NC	1
277	6	max	.014	2	.016	2	0	1	0	1	NC	4	NC	1
278		min	-.019	3	-.029	3	0	1	0	1	2121.861	3	NC	1
279	7	max	.013	2	.014	2	0	1	0	1	NC	4	NC	1
280		min	-.018	3	-.026	3	0	1	0	1	2300.046	3	NC	1
281	8	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
282		min	-.016	3	-.024	3	0	1	0	1	2510.775	3	NC	1
283	9	max	.011	2	.01	2	0	1	0	1	NC	1	NC	1
284		min	-.015	3	-.022	3	0	1	0	1	2763.819	3	NC	1
285	10	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
286		min	-.013	3	-.02	3	0	1	0	1	3073.293	3	NC	1
287	11	max	.009	2	.007	2	0	1	0	1	NC	1	NC	1
288		min	-.012	3	-.018	3	0	1	0	1	3460.373	3	NC	1
289	12	max	.007	2	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.01	3	-.015	3	0	1	0	1	3958.331	3	NC	1
291	13	max	.006	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.009	3	-.013	3	0	1	0	1	4622.625	3	NC	1
293	14	max	.005	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.007	3	-.011	3	0	1	0	1	5553.075	3	NC	1
295	15	max	.004	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.006	3	-.009	3	0	1	0	1	6949.326	3	NC	1
297	16	max	.003	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.007	3	0	1	0	1	9277.213	3	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.004	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.01	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.013	3	0	1	0	1	8116.36	3	NC	1
315	6	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.006	2	-.016	3	0	1	0	1	6840.101	3	NC	1
317	7	max	.007	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.007	2	-.018	3	0	1	0	1	6073.154	3	NC	1
319	8	max	.009	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.008	2	-.019	3	0	1	0	1	5613.825	3	NC	1
321	9	max	.01	3	-.005	15	0	1	0	1	NC	2	NC	1
322		min	-.009	2	-.02	3	0	1	0	1	5328.707	4	NC	1
323	10	max	.011	3	-.005	15	0	1	0	1	NC	2	NC	1
324		min	-.01	2	-.021	3	0	1	0	1	5120.617	4	NC	1
325	11	max	.012	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.011	2	-.021	3	0	1	0	1	5087.806	4	NC	1
327		12	max	.014	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.012	2	-.02	4	0	1	0	1	5229.48	4	NC	1
329		13	max	.015	3	-.004	15	0	1	0	1	NC	2	NC	1
330			min	-.013	2	-.019	4	0	1	0	1	5576.341	4	NC	1
331		14	max	.016	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.015	2	-.017	4	0	1	0	1	6206.906	4	NC	1
333		15	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
334			min	-.016	2	-.015	3	0	1	0	1	7296.774	4	NC	1
335		16	max	.019	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.017	2	-.012	3	0	1	0	1	9272.428	4	NC	1
337		17	max	.02	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.018	2	-.01	3	0	1	0	1	NC	1	NC	1
339		18	max	.021	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.019	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.022	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.02	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	2	.019	2	0	1	0	1	NC	1	NC	1
344			min	-.002	3	-.022	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	2	.018	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.021	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	2	.017	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.02	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	2	.016	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.019	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	2	.015	2	0	1	0	1	NC	1	NC	1
352			min	-.002	3	-.017	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	2	.014	2	0	1	0	1	NC	1	NC	1
354			min	-.002	3	-.016	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	2	.013	2	0	1	0	1	NC	1	NC	1
356			min	-.002	3	-.015	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	2	.012	2	0	1	0	1	NC	1	NC	1
358			min	-.001	3	-.014	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	2	.01	2	0	1	0	1	NC	1	NC	1
360			min	-.001	3	-.012	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	2	.009	2	0	1	0	1	NC	1	NC	1
362			min	-.001	3	-.011	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.008	2	0	1	0	1	NC	1	NC	1
364			min	-.001	3	-.01	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	2	.007	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	2	.006	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.005	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	2	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	2	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.009	2	0	15	9.914e-5	1	NC	1	NC	1
382			min	-.009	3	-.014	3	-.005	1	3.698e-6	15	6857.386	2	NC	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.006	2	.008	2	0	15	9.293e-5	1	NC	1	NC	1
384			min	-.008	3	-.013	3	-.004	1	3.467e-6	15	7820.882	2	NC	1
385		3	max	.006	2	.007	2	0	15	8.672e-5	1	NC	1	NC	1
386			min	-.008	3	-.013	3	-.004	1	3.237e-6	15	9084.039	2	NC	1
387		4	max	.005	2	.006	2	0	15	8.051e-5	1	NC	1	NC	1
388			min	-.007	3	-.012	3	-.004	1	3.006e-6	15	NC	1	NC	1
389		5	max	.005	2	.005	2	0	15	7.43e-5	1	NC	1	NC	1
390			min	-.007	3	-.012	3	-.003	1	2.775e-6	15	NC	1	NC	1
391		6	max	.005	2	.004	2	0	15	6.809e-5	1	NC	1	NC	1
392			min	-.006	3	-.011	3	-.003	1	2.545e-6	15	NC	1	NC	1
393		7	max	.004	2	.003	2	0	15	6.188e-5	1	NC	1	NC	1
394			min	-.006	3	-.01	3	-.002	1	2.314e-6	15	NC	1	NC	1
395		8	max	.004	2	.002	2	0	15	5.567e-5	1	NC	1	NC	1
396			min	-.005	3	-.01	3	-.002	1	2.083e-6	15	NC	1	NC	1
397		9	max	.004	2	.001	2	0	15	4.946e-5	1	NC	1	NC	1
398			min	-.005	3	-.009	3	-.002	1	1.841e-6	10	NC	1	NC	1
399		10	max	.003	2	0	2	0	15	4.325e-5	1	NC	1	NC	1
400			min	-.004	3	-.008	3	-.001	1	1.56e-6	10	NC	1	NC	1
401		11	max	.003	2	0	2	0	15	3.704e-5	1	NC	1	NC	1
402			min	-.004	3	-.008	3	-.001	1	1.28e-6	10	NC	1	NC	1
403		12	max	.003	2	0	2	0	15	3.083e-5	1	NC	1	NC	1
404			min	-.003	3	-.007	3	0	1	9.996e-7	10	NC	1	NC	1
405		13	max	.002	2	-.001	15	0	15	2.462e-5	1	NC	1	NC	1
406			min	-.003	3	-.006	3	0	1	7.193e-7	10	NC	1	NC	1
407		14	max	.002	2	0	15	0	15	1.842e-5	1	NC	1	NC	1
408			min	-.002	3	-.005	3	0	1	4.389e-7	10	NC	1	NC	1
409		15	max	.001	2	0	15	0	15	1.221e-5	1	NC	1	NC	1
410			min	-.002	3	-.004	3	0	1	1.586e-7	10	NC	1	NC	1
411		16	max	.001	2	0	15	0	15	5.996e-6	1	NC	1	NC	1
412			min	-.001	3	-.003	3	0	1	-1.218e-7	10	NC	1	NC	1
413		17	max	0	2	0	15	0	15	1.293e-6	3	NC	1	NC	1
414			min	0	3	-.002	3	0	1	-6.284e-7	2	NC	1	NC	1
415		18	max	0	2	0	15	0	15	1.196e-7	3	NC	1	NC	1
416			min	0	3	-.001	3	0	1	-6.423e-6	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	-4.534e-7	15	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.263e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	3.985e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	1.442e-7	15	NC	1	NC	1
421		2	max	0	3	0	15	0	15	-3.521e-7	15	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-9.556e-6	1	NC	1	NC	1
423		3	max	0	3	-.001	15	0	15	-8.484e-7	15	NC	1	NC	1
424			min	0	2	-.005	4	0	1	-2.31e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	0	15	-1.345e-6	15	NC	1	NC	1
426			min	-.001	2	-.008	4	0	1	-3.664e-5	1	NC	1	NC	1
427		5	max	.002	3	-.003	15	0	15	-1.841e-6	15	NC	1	NC	1
428			min	-.001	2	-.011	4	0	1	-5.018e-5	1	9263.067	4	NC	1
429		6	max	.002	3	-.003	15	0	15	-2.337e-6	15	NC	1	NC	1
430			min	-.002	2	-.014	4	0	1	-6.372e-5	1	7435.447	4	NC	1
431		7	max	.003	3	-.004	15	0	15	-2.834e-6	15	NC	5	NC	1
432			min	-.002	2	-.016	4	0	1	-7.726e-5	1	6338.796	4	NC	1
433		8	max	.003	3	-.004	15	0	15	-3.33e-6	15	NC	5	NC	1
434			min	-.002	2	-.018	4	0	1	-9.08e-5	1	5661.831	4	NC	1
435		9	max	.003	3	-.005	15	0	15	-3.826e-6	15	NC	5	NC	1
436			min	-.003	2	-.02	4	0	1	-1.043e-4	1	5258.368	4	NC	1
437		10	max	.004	3	-.005	15	0	15	-4.322e-6	15	NC	5	NC	1
438			min	-.003	2	-.021	4	-.001	1	-1.179e-4	1	5057.149	4	NC	1
439		11	max	.004	3	-.005	15	0	15	-4.819e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.004	2	-.021	4	-.001	1	-1.314e-4	1	5028.174	4	NC	1
441		max	.005	3	-.005	15	0	15	-5.315e-6	15	NC	5	NC	1
442		min	-.004	2	-.02	4	-.002	1	-1.45e-4	1	5171.142	4	NC	1
443		max	.005	3	-.004	15	0	15	-5.811e-6	15	NC	5	NC	1
444		min	-.004	2	-.019	4	-.002	1	-1.585e-4	1	5516.767	4	NC	1
445		max	.006	3	-.004	15	0	15	-6.308e-6	15	NC	5	NC	1
446		min	-.005	2	-.017	4	-.002	1	-1.72e-4	1	6143.024	4	NC	1
447		max	.006	3	-.003	15	0	15	-6.804e-6	15	NC	2	NC	1
448		min	-.005	2	-.015	4	-.003	1	-1.856e-4	1	7223.997	4	NC	1
449		max	.006	3	-.003	15	0	15	-7.3e-6	15	NC	1	NC	1
450		min	-.005	2	-.012	4	-.003	1	-1.991e-4	1	9182.26	4	NC	1
451		max	.007	3	-.002	15	0	15	-7.796e-6	15	NC	1	NC	1
452		min	-.006	2	-.008	4	-.003	1	-2.127e-4	1	NC	1	NC	1
453		max	.007	3	-.001	15	0	15	-8.293e-6	15	NC	1	NC	1
454		min	-.006	2	-.005	1	-.004	1	-2.262e-4	1	NC	1	NC	1
455		max	.008	3	0	15	0	15	-8.789e-6	15	NC	1	NC	1
456		min	-.006	2	-.002	1	-.004	1	-2.397e-4	1	NC	1	NC	1
457	M12	max	.003	2	.006	2	.004	1	-1.562e-6	15	NC	1	NC	2
458		min	0	3	-.008	3	0	15	-4.203e-5	1	NC	1	5577.399	1
459		max	.002	2	.006	2	.004	1	-1.562e-6	15	NC	1	NC	2
460		min	0	3	-.007	3	0	15	-4.203e-5	1	NC	1	6065.291	1
461		max	.002	2	.005	2	.004	1	-1.562e-6	15	NC	1	NC	2
462		min	0	3	-.007	3	0	15	-4.203e-5	1	NC	1	6645.943	1
463		max	.002	2	.005	2	.003	1	-1.562e-6	15	NC	1	NC	2
464		min	0	3	-.007	3	0	15	-4.203e-5	1	NC	1	7343.454	1
465		max	.002	2	.005	2	.003	1	-1.562e-6	15	NC	1	NC	2
466		min	0	3	-.006	3	0	15	-4.203e-5	1	NC	1	8190.573	1
467		max	.002	2	.004	2	.003	1	-1.562e-6	15	NC	1	NC	2
468		min	0	3	-.006	3	0	15	-4.203e-5	1	NC	1	9232.72	1
469		max	.002	2	.004	2	.002	1	-1.562e-6	15	NC	1	NC	1
470		min	0	3	-.005	3	0	15	-4.203e-5	1	NC	1	NC	1
471		max	.002	2	.004	2	.002	1	-1.562e-6	15	NC	1	NC	1
472		min	0	3	-.005	3	0	15	-4.203e-5	1	NC	1	NC	1
473		max	.001	2	.003	2	.002	1	-1.562e-6	15	NC	1	NC	1
474		min	0	3	-.004	3	0	15	-4.203e-5	1	NC	1	NC	1
475		max	.001	2	.003	2	.001	1	-1.562e-6	15	NC	1	NC	1
476		min	0	3	-.004	3	0	15	-4.203e-5	1	NC	1	NC	1
477		max	.001	2	.003	2	.001	1	-1.562e-6	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-4.203e-5	1	NC	1	NC	1
479		max	.001	2	.002	2	0	1	-1.562e-6	15	NC	1	NC	1
480		min	0	3	-.003	3	0	15	-4.203e-5	1	NC	1	NC	1
481		max	0	2	.002	2	0	1	-1.562e-6	15	NC	1	NC	1
482		min	0	3	-.003	3	0	15	-4.203e-5	1	NC	1	NC	1
483		max	0	2	.002	2	0	1	-1.562e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-4.203e-5	1	NC	1	NC	1
485		max	0	2	.001	2	0	1	-1.562e-6	15	NC	1	NC	1
486		min	0	3	-.002	3	0	15	-4.203e-5	1	NC	1	NC	1
487		max	0	2	0	2	0	1	-1.562e-6	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-4.203e-5	1	NC	1	NC	1
489		max	0	2	0	2	0	1	-1.562e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-4.203e-5	1	NC	1	NC	1
491		max	0	2	0	2	0	1	-1.562e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-4.203e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-1.562e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-4.203e-5	1	NC	1	NC	1
495	M1	max	.01	3	.245	2	0	1	5.315e-3	1	NC	1	NC	1
496		min	-.006	2	-.08	3	0	15	-1.398e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.01	3	.121	2	0	15	2.563e-3	1	NC	5	NC	1
498			min	-.006	2	-.04	3	-.003	1	-6.944e-3	3	1087.422	2	NC	1
499		3	max	.01	3	.014	3	0	15	2.969e-5	10	NC	5	NC	1
500			min	-.006	2	-.012	2	-.005	1	-9.02e-5	3	527.932	2	NC	1
501		4	max	.01	3	.093	3	0	15	3.706e-3	2	NC	15	NC	1
502			min	-.006	2	-.157	2	-.004	1	-3.662e-3	3	337.252	2	NC	1
503		5	max	.01	3	.189	3	0	15	7.391e-3	2	NC	15	NC	1
504			min	-.006	2	-.306	2	-.003	1	-7.235e-3	3	245.795	2	NC	1
505		6	max	.009	3	.291	3	0	10	1.108e-2	2	8735.955	15	NC	1
506			min	-.006	2	-.449	2	-.001	1	-1.081e-2	3	195.053	2	NC	1
507		7	max	.009	3	.387	3	0	1	1.476e-2	2	7404.42	15	NC	1
508			min	-.006	2	-.576	2	0	3	-1.438e-2	3	164.93	2	NC	1
509		8	max	.009	3	.466	3	0	1	1.845e-2	2	6613.918	15	NC	1
510			min	-.006	2	-.677	2	0	15	-1.795e-2	3	147.042	2	NC	1
511		9	max	.009	3	.518	3	0	15	2.053e-2	2	6198.955	15	NC	1
512			min	-.005	2	-.74	2	0	1	-1.859e-2	3	137.696	2	NC	1
513		10	max	.009	3	.537	3	0	1	2.153e-2	2	6071.719	15	NC	1
514			min	-.005	2	-.761	2	0	10	-1.728e-2	3	134.959	2	NC	1
515		11	max	.008	3	.525	3	0	1	2.254e-2	2	6198.557	15	NC	1
516			min	-.005	2	-.74	2	0	15	-1.596e-2	3	138.177	2	NC	1
517		12	max	.008	3	.481	3	0	15	2.143e-2	2	6613.052	15	NC	1
518			min	-.005	2	-.674	2	0	1	-1.405e-2	3	148.424	2	NC	1
519		13	max	.008	3	.411	3	0	10	1.718e-2	2	7402.874	15	NC	1
520			min	-.005	2	-.57	2	0	1	-1.124e-2	3	168.137	2	NC	1
521		14	max	.008	3	.32	3	.001	1	1.292e-2	2	8733.313	15	NC	1
522			min	-.005	2	-.439	2	0	15	-8.434e-3	3	201.677	2	NC	1
523		15	max	.008	3	.217	3	.003	1	8.671e-3	2	NC	15	NC	1
524			min	-.005	2	-.293	2	0	15	-5.627e-3	3	259.01	2	NC	1
525		16	max	.007	3	.11	3	.004	1	4.417e-3	2	NC	15	NC	1
526			min	-.005	2	-.145	2	0	15	-2.821e-3	3	364.197	2	NC	1
527		17	max	.007	3	.005	3	.004	1	3.201e-4	1	NC	5	NC	1
528			min	-.005	2	-.007	2	0	15	-1.392e-5	3	586.759	2	NC	1
529		18	max	.007	3	.112	2	.003	1	4.673e-3	2	NC	5	NC	1
530			min	-.005	2	-.091	3	0	15	-1.487e-3	3	1234.181	2	NC	1
531		19	max	.007	3	.219	2	0	15	9.341e-3	2	NC	1	NC	1
532			min	-.005	2	-.182	3	0	1	-3.036e-3	3	NC	1	NC	1
533	M5	1	max	.029	3	.349	2	0	1	0	1	NC	1	NC	1
534			min	-.02	2	-.003	3	0	1	0	1	NC	1	NC	1
535		2	max	.029	3	.172	2	0	1	0	1	NC	5	NC	1
536			min	-.02	2	-.004	3	0	1	0	1	779.799	2	NC	1
537		3	max	.029	3	.04	3	0	1	0	1	NC	5	NC	1
538			min	-.02	2	-.031	2	0	1	0	1	361.054	2	NC	1
539		4	max	.028	3	.165	3	0	1	0	1	NC	15	NC	1
540			min	-.019	2	-.283	2	0	1	0	1	216.665	2	NC	1
541		5	max	.027	3	.349	3	0	1	0	1	7477.776	15	NC	1
542			min	-.019	2	-.562	2	0	1	0	1	150.007	2	NC	1
543		6	max	.027	3	.561	3	0	1	0	1	5707.854	15	NC	1
544			min	-.019	2	-.843	2	0	1	0	1	114.541	2	NC	1
545		7	max	.026	3	.771	3	0	1	0	1	4694.977	15	NC	1
546			min	-.018	2	-1.1	2	0	1	0	1	94.198	2	NC	1
547		8	max	.026	3	.949	3	0	1	0	1	4110.161	15	NC	1
548			min	-.018	2	-1.307	2	0	1	0	1	82.432	2	NC	1
549		9	max	.025	3	1.064	3	0	1	0	1	3811.395	15	NC	1
550			min	-.018	2	-1.439	2	0	1	0	1	76.415	2	NC	1
551		10	max	.024	3	1.106	3	0	1	0	1	3721.492	15	NC	1
552			min	-.017	2	-1.485	2	0	1	0	1	74.658	2	NC	1
553		11	max	.024	3	1.078	3	0	1	0	1	3811.695	15	NC	1





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 23, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.017	2	-1.441	2	0	1	0	1	76.713	2	NC	1
555	12	max	.023	3	.982	3	0	1	0	1	4110.853	15	NC	1
556		min	-.017	2	-1.304	2	0	1	0	1	83.441	2	NC	1
557	13	max	.023	3	.828	3	0	1	0	1	4696.324	15	NC	1
558		min	-.016	2	-1.085	2	0	1	0	1	96.896	2	NC	1
559	14	max	.022	3	.635	3	0	1	0	1	5710.394	15	NC	1
560		min	-.016	2	-.816	2	0	1	0	1	120.816	2	NC	1
561	15	max	.021	3	.422	3	0	1	0	1	7482.689	15	NC	1
562		min	-.016	2	-.527	2	0	1	0	1	164.211	2	NC	1
563	16	max	.021	3	.208	3	0	1	0	1	NC	15	NC	1
564		min	-.015	2	-.25	2	0	1	0	1	250.378	2	NC	1
565	17	max	.02	3	.013	3	0	1	0	1	NC	5	NC	1
566		min	-.015	2	-.017	2	0	1	0	1	449.303	2	NC	1
567	18	max	.02	3	.147	2	0	1	0	1	NC	5	NC	1
568		min	-.015	2	-.148	3	0	1	0	1	1031.967	2	NC	1
569	19	max	.02	3	.273	2	0	1	0	1	NC	1	NC	1
570		min	-.015	2	-.288	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.245	2	0	15	1.398e-2	3	NC	1	NC	1
572		min	-.006	2	-.08	3	0	1	-5.315e-3	1	NC	1	NC	1
573	2	max	.01	3	.121	2	.003	1	6.944e-3	3	NC	5	NC	1
574		min	-.006	2	-.04	3	0	15	-2.563e-3	1	1087.422	2	NC	1
575	3	max	.01	3	.014	3	.005	1	9.02e-5	3	NC	5	NC	1
576		min	-.006	2	-.012	2	0	15	-2.969e-5	10	527.932	2	NC	1
577	4	max	.01	3	.093	3	.004	1	3.662e-3	3	NC	15	NC	1
578		min	-.006	2	-.157	2	0	15	-3.706e-3	2	337.252	2	NC	1
579	5	max	.01	3	.189	3	.003	1	7.235e-3	3	NC	15	NC	1
580		min	-.006	2	-.306	2	0	15	-7.391e-3	2	245.795	2	NC	1
581	6	max	.009	3	.291	3	.001	1	1.081e-2	3	8735.955	15	NC	1
582		min	-.006	2	-.449	2	0	10	-1.108e-2	2	195.053	2	NC	1
583	7	max	.009	3	.387	3	0	3	1.438e-2	3	7404.42	15	NC	1
584		min	-.006	2	-.576	2	0	1	-1.476e-2	2	164.93	2	NC	1
585	8	max	.009	3	.466	3	0	15	1.795e-2	3	6613.918	15	NC	1
586		min	-.006	2	-.677	2	0	1	-1.845e-2	2	147.042	2	NC	1
587	9	max	.009	3	.518	3	0	1	1.859e-2	3	6198.955	15	NC	1
588		min	-.005	2	-.74	2	0	15	-2.053e-2	2	137.696	2	NC	1
589	10	max	.009	3	.537	3	0	10	1.728e-2	3	6071.719	15	NC	1
590		min	-.005	2	-.761	2	0	1	-2.153e-2	2	134.959	2	NC	1
591	11	max	.008	3	.525	3	0	15	1.596e-2	3	6198.557	15	NC	1
592		min	-.005	2	-.74	2	0	1	-2.254e-2	2	138.177	2	NC	1
593	12	max	.008	3	.481	3	0	1	1.405e-2	3	6613.052	15	NC	1
594		min	-.005	2	-.674	2	0	15	-2.143e-2	2	148.424	2	NC	1
595	13	max	.008	3	.411	3	0	1	1.124e-2	3	7402.874	15	NC	1
596		min	-.005	2	-.57	2	0	10	-1.718e-2	2	168.137	2	NC	1
597	14	max	.008	3	.32	3	0	15	8.434e-3	3	8733.313	15	NC	1
598		min	-.005	2	-.439	2	-.001	1	-1.292e-2	2	201.677	2	NC	1
599	15	max	.008	3	.217	3	0	15	5.627e-3	3	NC	15	NC	1
600		min	-.005	2	-.293	2	-.003	1	-8.671e-3	2	259.01	2	NC	1
601	16	max	.007	3	.11	3	0	15	2.821e-3	3	NC	15	NC	1
602		min	-.005	2	-.145	2	-.004	1	-4.417e-3	2	364.197	2	NC	1
603	17	max	.007	3	.005	3	0	15	1.392e-5	3	NC	5	NC	1
604		min	-.005	2	-.007	2	-.004	1	-3.201e-4	1	586.759	2	NC	1
605	18	max	.007	3	.112	2	0	15	1.487e-3	3	NC	5	NC	1
606		min	-.005	2	-.091	3	-.003	1	-4.673e-3	2	1234.181	2	NC	1
607	19	max	.007	3	.219	2	0	1	3.036e-3	3	NC	1	NC	1
608		min	-.005	2	-.182	3	0	15	-9.341e-3	2	NC	1	NC	1



Anchor Designer™  
Software  
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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

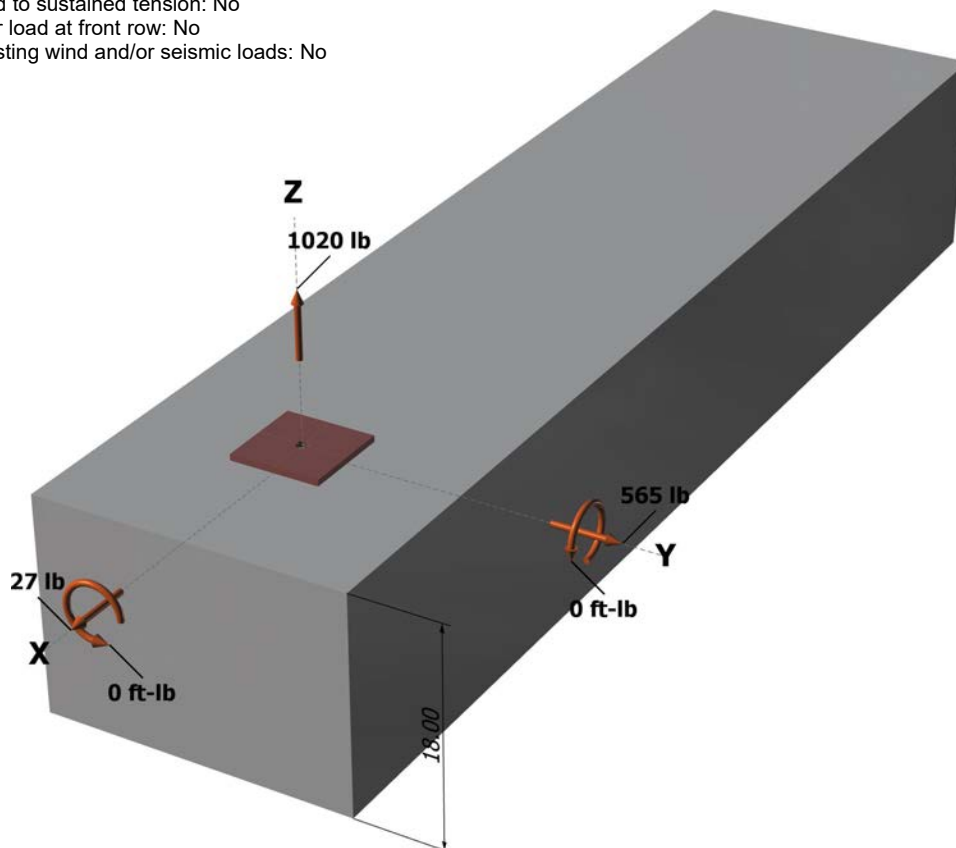
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





**Anchor Designer™**  
Software  
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### 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain ( $\epsilon_o$ ): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1020  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



### 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

### 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

### 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

## 2. Input Data & Anchor Parameters

## General

Design method:ACI 318-05  
Units: Imperial units

**Anchor Information:**

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

## Load and Geometry

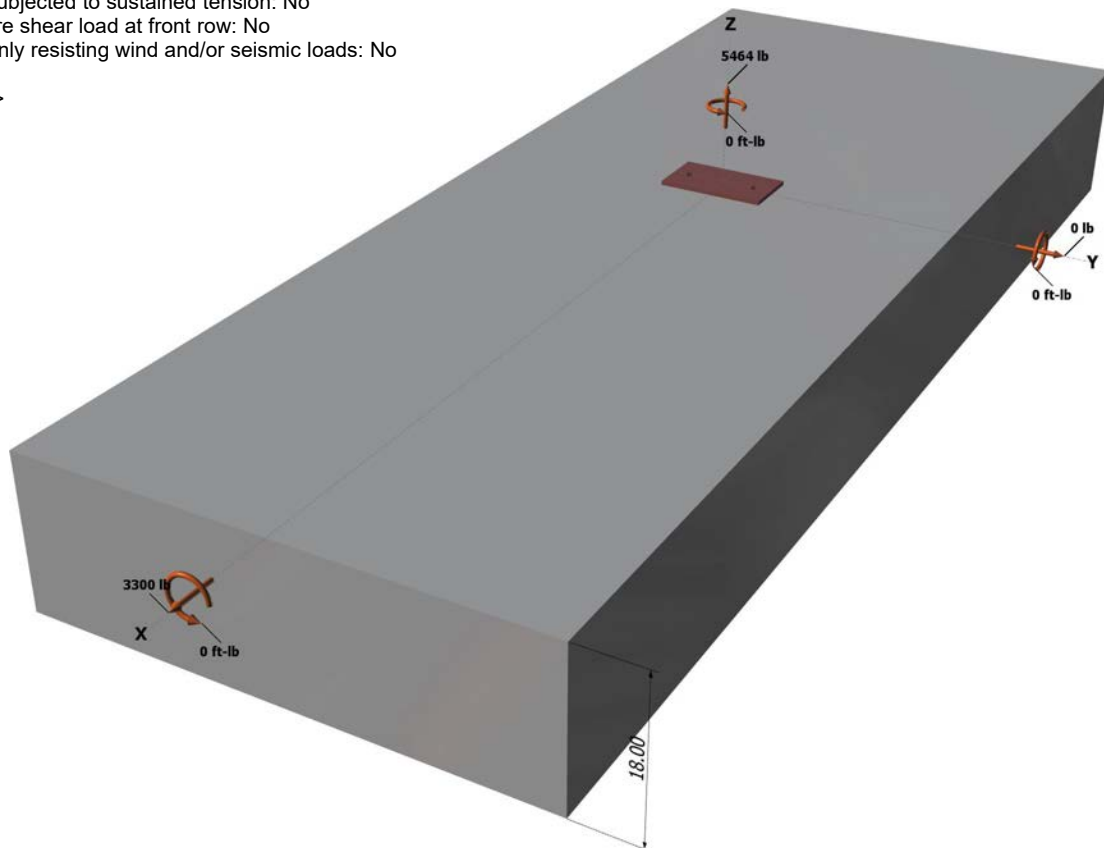
Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

### Base Material

Concrete: Normal-weight  
Concrete thickness, h (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

## Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28



<Figure 1>

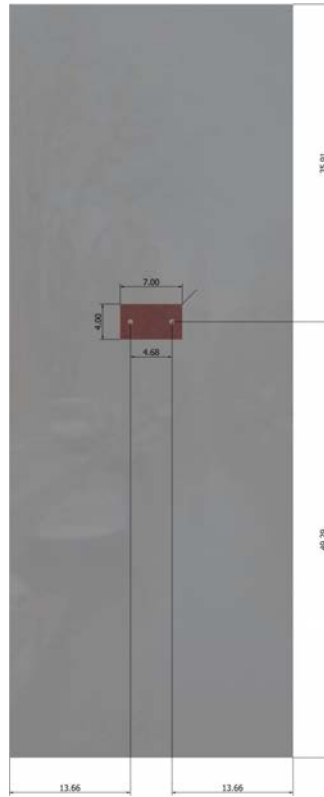
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







# Anchor Designer™ Software Version 2.4.6025.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.