

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	25° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

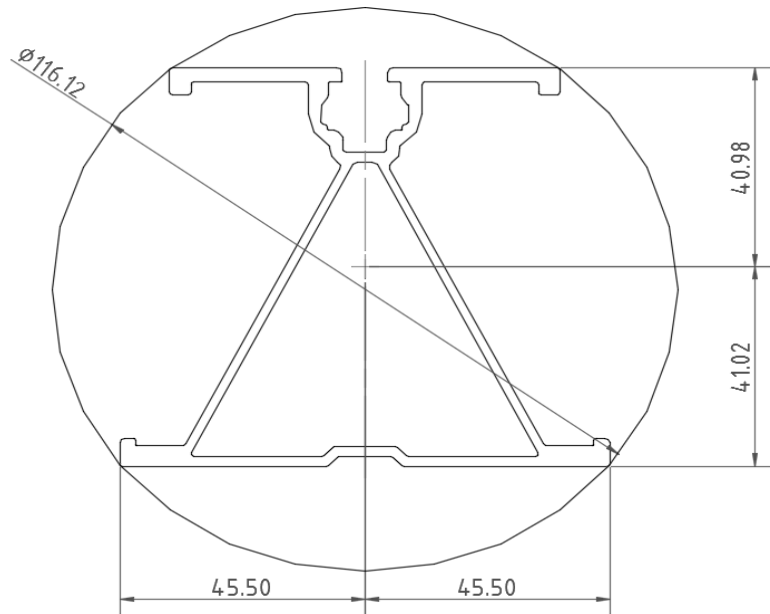
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	81 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	-1.775 k-ft
M_z =	-0.005 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	64%

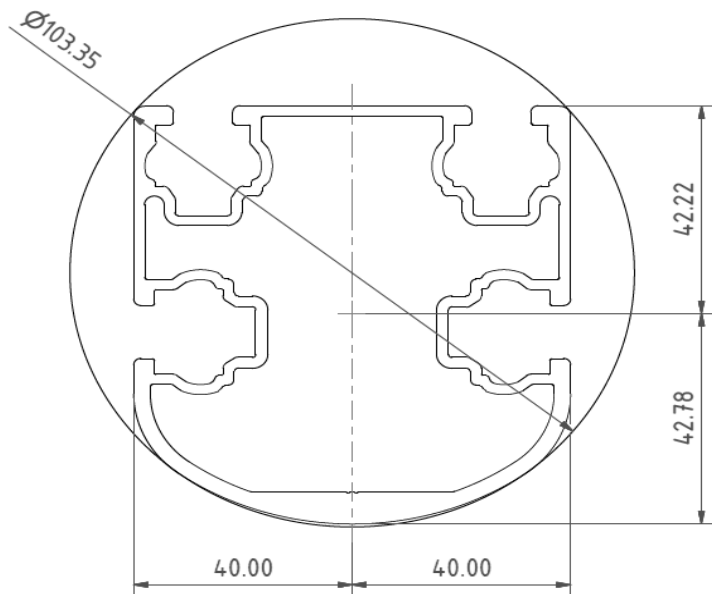


DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.361 k-ft
M_z =	0.000 k-ft
P_n =	-0.964 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	100%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	2.833 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>10%</u>



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.013 k-ft
M_z =	0.000 k-ft
P_n =	2.449 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<u>42%</u>



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	69.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.82 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.013 k-ft
M_z =	0.000 k-ft
P_n =	3.358 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.629 k
Utilization =	<u>33%</u>



5. FOUNDATION DESIGN CALCULATIONS

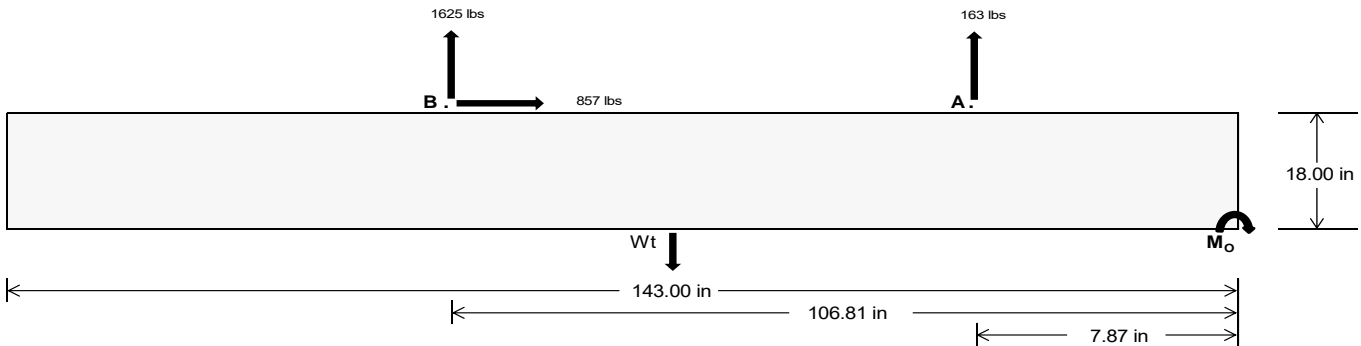
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>723.55</u>	<u>7055.72</u>	k
Compressive Load =	<u>3683.40</u>	<u>5141.44</u>	k
Lateral Load =	<u>9.50</u>	<u>3711.43</u>	k
Moment (Weak Axis) =	<u>0.02</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 190293.0$ in-lbs
Resisting Force Required = 2661.44 lbs
S.F. = 1.67
Weight Required = 4435.74 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 856.70 lbs
Friction = 0.4
Weight Required = 2141.75 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 856.70 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$ 7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	1063 lbs	1063 lbs	1063 lbs	1063 lbs	1497 lbs	1497 lbs	1497 lbs	1497 lbs	1820 lbs	1820 lbs	1820 lbs	1820 lbs	-326 lbs	-326 lbs	-326 lbs	-326 lbs
F_B	1084 lbs	1084 lbs	1084 lbs	1084 lbs	2164 lbs	2164 lbs	2164 lbs	2164 lbs	2335 lbs	2335 lbs	2335 lbs	2335 lbs	-3250 lbs	-3250 lbs	-3250 lbs	-3250 lbs
F_V	113 lbs	113 lbs	113 lbs	113 lbs	1523 lbs	1523 lbs	1523 lbs	1523 lbs	1217 lbs	1217 lbs	1217 lbs	1217 lbs	-1713 lbs	-1713 lbs	-1713 lbs	-1713 lbs
P_{total}	9707 lbs	9923 lbs	10139 lbs	10355 lbs	11221 lbs	11437 lbs	11653 lbs	11869 lbs	11714 lbs	11930 lbs	12146 lbs	12362 lbs	959 lbs	1089 lbs	1218 lbs	1348 lbs
M	2612 lbs-ft	2612 lbs-ft	2612 lbs-ft	2612 lbs-ft	3856 lbs-ft	3856 lbs-ft	3856 lbs-ft	3856 lbs-ft	4604 lbs-ft	4604 lbs-ft	4604 lbs-ft	4604 lbs-ft	5264 lbs-ft	5264 lbs-ft	5264 lbs-ft	5264 lbs-ft
e	0.27 ft	0.26 ft	0.26 ft	0.25 ft	0.34 ft	0.34 ft	0.33 ft	0.32 ft	0.39 ft	0.39 ft	0.38 ft	0.37 ft	5.49 ft	4.84 ft	4.32 ft	3.91 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	241.4 psf	240.8 psf	240.1 psf	239.5 psf	267.0 psf	265.6 psf	264.3 psf	263.1 psf	270.3 psf	268.9 psf	267.5 psf	266.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	317.1 psf	314.3 psf	311.7 psf	309.2 psf	378.7 psf	374.2 psf	370.0 psf	366.0 psf	403.7 psf	398.6 psf	393.7 psf	389.0 psf	466.8 psf	215.4 psf	160.9 psf	138.2 psf

Maximum Bearing Pressure = 467 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

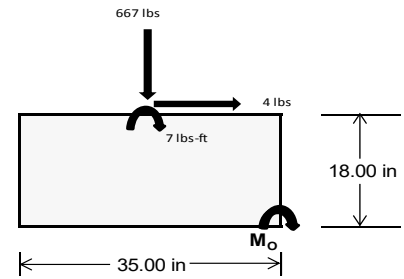
Overturning Check

$M_o = 960.2 \text{ ft-lbs}$
 Resisting Force Required = 658.40 lbs
 S.F. = 1.67
 Weight Required = 1097.33 lbs
 Minimum Width = 35 in
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	214 lbs	500 lbs	214 lbs	667 lbs	1783 lbs	667 lbs	62 lbs	146 lbs	62 lbs
F_v	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
P_{total}	9573 lbs	7560 lbs	9573 lbs	9576 lbs	7560 lbs	9576 lbs	2799 lbs	7560 lbs	2799 lbs
M	3 lbs-ft	0 lbs-ft	3 lbs-ft	13 lbs-ft	0 lbs-ft	13 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	275.2 psf	217.5 psf	275.2 psf	274.8 psf	217.5 psf	274.8 psf	80.5 psf	217.5 psf	80.5 psf
f_{max}	275.6 psf	217.5 psf	275.6 psf	276.3 psf	217.5 psf	276.3 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 276 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 35in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	1.118 k
Allowable Uplift =	1.214 k
Utilization =	<u>92%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.703 k
Allowable Uplift =	4.357 k
Utilization =	<u>62%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.833 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

Rear Strut

Maximum Axial Load =	4.806 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>65%</u>

Diagonal Strut

Maximum Axial Load =	2.640 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>36%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	56.48 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.130 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 81 \text{ in}$$

$$J = 0.432$$

$$224.084$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 81$$

$$J = 0.432$$

$$142.504$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$C_c = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$C_c = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 69.80 \text{ in} \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 69.8 \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

Compression

3.4.7

$$\begin{aligned} \lambda &= 1.61471 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.80606 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 10.8205 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

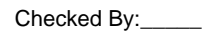
3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 10.82 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 11.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMax 72 Cell 2V 25° 160mph 30psf 6.75ft 7-10 Page 3 of 3



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 25° 160mph 30psf 6.75ft 7-10PaSe3d



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	43.299	1	235.425	2	2.846	3	.012	2	-.003	15	.836	3
28			min	1.817	15	-384.44	3	-26.224	1	0	15	-.084	1	-.4	2
29		15	max	43.299	1	98.622	2	6.074	1	.012	2	-.003	12	1.034	3
30			min	1.817	15	-141.437	3	-1.051	10	0	15	-.092	1	-.525	2
31		16	max	43.299	1	101.565	3	38.372	1	.012	2	0	3	1.049	3
32			min	1.817	15	-38.18	2	1.611	15	0	15	-.075	1	-.548	2
33		17	max	43.299	1	344.568	3	70.67	1	.012	2	.005	3	.881	3
34			min	1.817	15	-174.982	2	2.913	15	0	15	-.034	1	-.468	2
35		18	max	43.299	1	587.57	3	102.968	1	.012	2	.031	1	.532	3
36			min	1.817	15	-311.784	2	4.215	15	0	15	0	10	-.285	2
37		19	max	43.299	1	830.573	3	135.266	1	.012	2	.12	1	0	2
38			min	1.817	15	-448.587	2	5.517	15	0	15	.005	15	0	3
39	M14	1	max	30.261	1	546.794	2	-5.783	15	.016	3	.149	1	0	1
40			min	1.262	15	-686.673	3	-141.762	1	-.017	2	.006	15	0	3
41		2	max	30.261	1	409.992	2	-4.481	15	.016	3	.055	1	.446	3
42			min	1.262	15	-503.013	3	-109.464	1	-.017	2	.001	10	-.359	2
43		3	max	30.261	1	273.19	2	-3.179	15	.016	3	.008	3	.755	3
44			min	1.262	15	-319.353	3	-77.166	1	-.017	2	-.015	1	-.615	2
45		4	max	30.261	1	136.388	2	-1.877	15	.016	3	.001	3	.925	3
46			min	1.262	15	-135.692	3	-44.868	1	-.017	2	-.061	1	-.769	2
47		5	max	30.261	1	47.968	3	.448	10	.016	3	-.002	12	.958	3
48			min	1.262	15	-5.33	1	-12.57	1	-.017	2	-.082	1	-.82	2
49		6	max	30.261	1	231.628	3	19.728	1	.016	3	-.003	15	.853	3
50			min	1.262	15	-137.217	2	-3.41	3	-.017	2	-.079	1	-.768	2
51		7	max	30.261	1	415.289	3	52.026	1	.016	3	-.002	15	.611	3
52			min	1.262	15	-274.019	2	-1.425	3	-.017	2	-.052	1	-.614	2
53		8	max	30.261	1	598.949	3	84.324	1	.016	3	.005	2	.23	3
54			min	1.262	15	-410.821	2	.56	3	-.017	2	-.009	3	-.357	2
55		9	max	30.261	1	782.609	3	116.622	1	.016	3	.074	1	.032	1
56			min	1.262	15	-547.624	2	1.992	12	-.017	2	-.008	3	-.288	3
57		10	max	30.261	1	966.269	3	148.92	1	.017	2	.174	1	.465	2
58			min	1.262	15	-684.426	2	3.315	12	-.016	3	-.005	3	-.944	3
59		11	max	30.261	1	547.624	2	-1.992	12	.017	2	.074	1	.032	1
60			min	1.262	15	-782.609	3	-116.622	1	-.016	3	-.008	3	-.288	3
61		12	max	30.261	1	410.821	2	-.56	3	.017	2	.005	2	.23	3
62			min	1.262	15	-598.949	3	-84.324	1	-.016	3	-.009	3	-.357	2
63		13	max	30.261	1	274.019	2	1.425	3	.017	2	-.002	15	.611	3
64			min	1.262	15	-415.289	3	-52.026	1	-.016	3	-.052	1	-.614	2
65		14	max	30.261	1	137.217	2	3.41	3	.017	2	-.003	15	.853	3
66			min	1.262	15	-231.628	3	-19.728	1	-.016	3	-.079	1	-.768	2
67		15	max	30.261	1	5.33	1	12.57	1	.017	2	-.002	12	.958	3
68			min	1.262	15	-47.968	3	-.448	10	-.016	3	-.082	1	-.82	2
69		16	max	30.261	1	135.692	3	44.868	1	.017	2	.001	3	.925	3
70			min	1.262	15	-136.388	2	1.877	15	-.016	3	-.061	1	-.769	2
71		17	max	30.261	1	319.353	3	77.166	1	.017	2	.008	3	.755	3
72			min	1.262	15	-273.19	2	3.179	15	-.016	3	-.015	1	-.615	2
73		18	max	30.261	1	503.013	3	109.464	1	.017	2	.055	1	.446	3
74			min	1.262	15	-409.992	2	4.481	15	-.016	3	.001	10	-.359	2
75		19	max	30.261	1	686.673	3	141.762	1	.017	2	.149	1	0	1
76			min	1.262	15	-546.794	2	5.783	15	-.016	3	.006	15	0	3
77	M15	1	max	-1.328	15	747.299	2	-5.78	15	.018	2	.149	1	0	2
78			min	-31.542	1	-387.672	3	-141.798	1	-.013	3	.006	15	0	3
79		2	max	-1.328	15	551.154	2	-4.478	15	.018	2	.055	1	.255	3
80			min	-31.542	1	-293.025	3	-109.5	1	-.013	3	.002	10	-.487	2
81		3	max	-1.328	15	355.01	2	-3.176	15	.018	2	.007	3	.44	3
82			min	-31.542	1	-198.378	3	-77.202	1	-.013	3	-.015	1	-.827	2
83		4	max	-1.328	15	158.865	2	-1.874	15	.018	2	0	3	.553	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-31.542	1	-103.73	3	-44.904	1	-.013	3	-.061	1	-1.019	2
85		5	max	-1.328	15	-.274	15	.33	10	.018	2	-.003	12	.595	3
86			min	-31.542	1	-37.279	2	-12.606	1	-.013	3	-.082	1	-1.065	2
87		6	max	-1.328	15	85.564	3	19.692	1	.018	2	-.003	15	.566	3
88			min	-31.542	1	-233.424	2	-3.017	3	-.013	3	-.079	1	-.964	2
89		7	max	-1.328	15	180.212	3	51.99	1	.018	2	-.002	15	.467	3
90			min	-31.542	1	-429.568	2	-1.032	3	-.013	3	-.053	1	-.715	2
91		8	max	-1.328	15	274.859	3	84.288	1	.018	2	.004	2	.296	3
92			min	-31.542	1	-625.713	2	.905	12	-.013	3	-.008	3	-.319	2
93		9	max	-1.328	15	369.506	3	116.586	1	.018	2	.074	1	.224	2
94			min	-31.542	1	-821.857	2	2.229	12	-.013	3	-.007	3	.002	15
95		10	max	-1.328	15	464.153	3	148.884	1	.013	3	.173	1	.914	2
96			min	-31.542	1	-1018.002	2	3.552	12	-.018	2	-.004	3	-.258	3
97		11	max	-1.328	15	821.857	2	-2.229	12	.013	3	.074	1	.224	2
98			min	-31.542	1	-369.506	3	-116.586	1	-.018	2	-.007	3	.002	15
99		12	max	-1.328	15	625.713	2	-.905	12	.013	3	.004	2	.296	3
100			min	-31.542	1	-274.859	3	-84.288	1	-.018	2	-.008	3	-.319	2
101		13	max	-1.328	15	429.568	2	1.032	3	.013	3	-.002	15	.467	3
102			min	-31.542	1	-180.212	3	-51.99	1	-.018	2	-.053	1	-.715	2
103		14	max	-1.328	15	233.424	2	3.017	3	.013	3	-.003	15	.566	3
104			min	-31.542	1	-85.564	3	-19.692	1	-.018	2	-.079	1	-.964	2
105		15	max	-1.328	15	37.279	2	12.606	1	.013	3	-.003	12	.595	3
106			min	-31.542	1	.274	15	-.33	10	-.018	2	-.082	1	-1.065	2
107		16	max	-1.328	15	103.73	3	44.904	1	.013	3	0	3	.553	3
108			min	-31.542	1	-158.865	2	1.874	15	-.018	2	-.061	1	-1.019	2
109		17	max	-1.328	15	198.378	3	77.202	1	.013	3	.007	3	.44	3
110			min	-31.542	1	-355.01	2	3.176	15	-.018	2	-.015	1	-.827	2
111		18	max	-1.328	15	293.025	3	109.5	1	.013	3	.055	1	.255	3
112			min	-31.542	1	-551.154	2	4.478	15	-.018	2	.002	10	-.487	2
113		19	max	-1.328	15	387.672	3	141.798	1	.013	3	.149	1	0	2
114			min	-31.542	1	-747.299	2	5.78	15	-.018	2	.006	15	0	3
115	M16	1	max	-2.042	15	654.611	2	-5.531	15	.006	1	.123	1	0	2
116			min	-48.89	1	-305.07	3	-135.953	1	-.012	3	.005	15	0	3
117		2	max	-2.042	15	458.466	2	-4.229	15	.006	1	.033	1	.193	3
118			min	-48.89	1	-210.423	3	-103.655	1	-.012	3	0	10	-.417	2
119		3	max	-2.042	15	262.322	2	-2.927	15	.006	1	.003	3	.316	3
120			min	-48.89	1	-115.776	3	-71.357	1	-.012	3	-.033	1	-.688	2
121		4	max	-2.042	15	66.177	2	-1.625	15	.006	1	-.001	12	.367	3
122			min	-48.89	1	-21.129	3	-39.058	1	-.012	3	-.074	1	-.811	2
123		5	max	-2.042	15	73.519	3	.612	10	.006	1	-.003	12	.347	3
124			min	-48.89	1	-129.967	2	-6.76	1	-.012	3	-.091	1	-.787	2
125		6	max	-2.042	15	168.166	3	25.538	1	.006	1	-.003	15	.257	3
126			min	-48.89	1	-326.112	2	-1.616	3	-.012	3	-.084	1	-.616	2
127		7	max	-2.042	15	262.813	3	57.836	1	.006	1	-.002	15	.095	3
128			min	-48.89	1	-522.256	2	.369	3	-.012	3	-.053	1	-.298	2
129		8	max	-2.042	15	357.461	3	90.134	1	.006	1	.005	2	.167	2
130			min	-48.89	1	-718.401	2	1.772	12	-.012	3	-.007	3	-.138	3
131		9	max	-2.042	15	452.108	3	122.432	1	.006	1	.082	1	.78	2
132			min	-48.89	1	-914.545	2	3.096	12	-.012	3	-.004	3	-.441	3
133		10	max	-2.042	15	546.755	3	154.73	1	.012	3	.186	1	1.539	2
134			min	-48.89	1	-1110.69	2	4.419	12	-.006	1	0	3	-.816	3
135		11	max	-2.042	15	914.545	2	-3.096	12	.012	3	.082	1	.78	2
136			min	-48.89	1	-452.108	3	-122.432	1	-.006	1	-.004	3	-.441	3
137		12	max	-2.042	15	718.401	2	-1.772	12	.012	3	.005	2	.167	2
138			min	-48.89	1	-357.461	3	-90.134	1	-.006	1	-.007	3	-.138	3
139		13	max	-2.042	15	522.256	2	-.369	3	.012	3	-.002	15	.095	3
140			min	-48.89	1	-262.813	3	-57.836	1	-.006	1	-.053	1	-.298	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-2.042	15	326.112	2	1.616	3	.012	3	-.003	15	.257	3
142		min	-48.89	1	-168.166	3	-25.538	1	-.006	1	-.084	1	-.616	2
143	15	max	-2.042	15	129.967	2	6.76	1	.012	3	-.003	12	.347	3
144		min	-48.89	1	-73.519	3	-.612	10	-.006	1	-.091	1	-.787	2
145	16	max	-2.042	15	21.129	3	39.058	1	.012	3	-.001	12	.367	3
146		min	-48.89	1	-66.177	2	1.625	15	-.006	1	-.074	1	-.811	2
147	17	max	-2.042	15	115.776	3	71.357	1	.012	3	.003	3	.316	3
148		min	-48.89	1	-262.322	2	2.927	15	-.006	1	-.033	1	-.688	2
149	18	max	-2.042	15	210.423	3	103.655	1	.012	3	.033	1	.193	3
150		min	-48.89	1	-458.466	2	4.229	15	-.006	1	0	10	-.417	2
151	19	max	-2.042	15	305.07	3	135.953	1	.012	3	.123	1	0	2
152		min	-48.89	1	-654.611	2	5.531	15	-.006	1	.005	15	0	3
153	M2	1	max	1135.679	2	2.025	4	.342	1	0	3	0	3	1
154		min	-1578.237	3	.476	15	.014	15	0	1	0	2	0	1
155	2	max	1136.153	2	1.988	4	.342	1	0	3	0	1	0	15
156		min	-1577.881	3	.467	15	.014	15	0	1	0	10	0	4
157	3	max	1136.627	2	1.951	4	.342	1	0	3	0	1	0	15
158		min	-1577.526	3	.459	15	.014	15	0	1	0	15	-.001	4
159	4	max	1137.1	2	1.914	4	.342	1	0	3	0	1	0	15
160		min	-1577.171	3	.45	15	.014	15	0	1	0	15	-.002	4
161	5	max	1137.574	2	1.877	4	.342	1	0	3	0	1	0	15
162		min	-1576.815	3	.441	15	.014	15	0	1	0	15	-.002	4
163	6	max	1138.048	2	1.84	4	.342	1	0	3	0	1	0	15
164		min	-1576.46	3	.433	15	.014	15	0	1	0	15	-.003	4
165	7	max	1138.522	2	1.803	4	.342	1	0	3	0	1	0	15
166		min	-1576.105	3	.424	15	.014	15	0	1	0	15	-.004	4
167	8	max	1138.995	2	1.766	4	.342	1	0	3	0	1	0	15
168		min	-1575.749	3	.415	15	.014	15	0	1	0	15	-.004	4
169	9	max	1139.469	2	1.729	4	.342	1	0	3	0	1	-.001	15
170		min	-1575.394	3	.406	15	.014	15	0	1	0	15	-.005	4
171	10	max	1139.943	2	1.692	4	.342	1	0	3	0	1	-.001	15
172		min	-1575.039	3	.398	15	.014	15	0	1	0	15	-.005	4
173	11	max	1140.417	2	1.655	4	.342	1	0	3	.001	1	-.001	15
174		min	-1574.684	3	.389	15	.014	15	0	1	0	15	-.006	4
175	12	max	1140.89	2	1.618	4	.342	1	0	3	.001	1	-.002	15
176		min	-1574.328	3	.38	15	.014	15	0	1	0	15	-.006	4
177	13	max	1141.364	2	1.581	4	.342	1	0	3	.001	1	-.002	15
178		min	-1573.973	3	.372	15	.014	15	0	1	0	15	-.007	4
179	14	max	1141.838	2	1.544	4	.342	1	0	3	.001	1	-.002	15
180		min	-1573.618	3	.363	15	.014	15	0	1	0	15	-.007	4
181	15	max	1142.312	2	1.507	4	.342	1	0	3	.002	1	-.002	15
182		min	-1573.262	3	.354	15	.014	15	0	1	0	15	-.008	4
183	16	max	1142.785	2	1.47	4	.342	1	0	3	.002	1	-.002	15
184		min	-1572.907	3	.346	15	.014	15	0	1	0	15	-.008	4
185	17	max	1143.259	2	1.433	4	.342	1	0	3	.002	1	-.002	15
186		min	-1572.552	3	.332	12	.014	15	0	1	0	15	-.009	4
187	18	max	1143.733	2	1.396	4	.342	1	0	3	.002	1	-.002	15
188		min	-1572.196	3	.318	12	.014	15	0	1	0	15	-.009	4
189	19	max	1144.207	2	1.359	4	.342	1	0	3	.002	1	-.002	15
190		min	-1571.841	3	.303	12	.014	15	0	1	0	15	-.01	4
191	M3	1	max	773.217	2	8.995	4	.164	1	0	5	0	.01	4
192		min	-910.32	3	2.114	15	.007	15	0	1	0	15	.002	15
193	2	max	773.047	2	8.122	4	.164	1	0	5	0	1	.006	2
194		min	-910.447	3	1.909	15	.007	15	0	1	0	15	0	12
195	3	max	772.876	2	7.25	4	.164	1	0	5	0	1	.003	2
196		min	-910.575	3	1.704	15	.007	15	0	1	0	15	0	3
197	4	max	772.706	2	6.378	4	.164	1	0	5	0	1	0	2





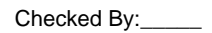
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1008.596	1	0	1	-.312	15	0	1	0	15	0	1
256		min	-155.844	3	0	1	-7.525	1	0	1	-.01	1	0	1
257	15	max	1008.766	1	0	1	-.312	15	0	1	0	15	0	1
258		min	-155.716	3	0	1	-7.525	1	0	1	-.011	1	0	1
259	16	max	1008.936	1	0	1	-.312	15	0	1	0	15	0	1
260		min	-155.588	3	0	1	-7.525	1	0	1	-.012	1	0	1
261	17	max	1009.107	1	0	1	-.312	15	0	1	0	15	0	1
262		min	-155.461	3	0	1	-7.525	1	0	1	-.013	1	0	1
263	18	max	1009.277	1	0	1	-.312	15	0	1	0	15	0	1
264		min	-155.333	3	0	1	-7.525	1	0	1	-.014	1	0	1
265	19	max	1009.447	1	0	1	-.312	15	0	1	0	15	0	1
266		min	-155.205	3	0	1	-7.525	1	0	1	-.015	1	0	1
267	M6	1	max	3349.706	2	2.436	2	0	1	0	0	1	0	1
268		min	-4806.133	3	.073	3	0	1	0	1	0	1	0	1
269	2	max	3350.18	2	2.407	2	0	1	0	1	0	1	0	3
270		min	-4805.778	3	.052	3	0	1	0	1	0	1	0	2
271	3	max	3350.654	2	2.378	2	0	1	0	1	0	1	0	3
272		min	-4805.422	3	.03	3	0	1	0	1	0	1	-.002	2
273	4	max	3351.127	2	2.349	2	0	1	0	1	0	1	0	3
274		min	-4805.067	3	.008	3	0	1	0	1	0	1	-.002	2
275	5	max	3351.601	2	2.32	2	0	1	0	1	0	1	0	3
276		min	-4804.712	3	-.013	3	0	1	0	1	0	1	-.003	2
277	6	max	3352.075	2	2.291	2	0	1	0	1	0	1	0	3
278		min	-4804.357	3	-.035	3	0	1	0	1	0	1	-.004	2
279	7	max	3352.549	2	2.263	2	0	1	0	1	0	1	0	3
280		min	-4804.001	3	-.057	3	0	1	0	1	0	1	-.005	2
281	8	max	3353.022	2	2.234	2	0	1	0	1	0	1	0	3
282		min	-4803.646	3	-.078	3	0	1	0	1	0	1	-.005	2
283	9	max	3353.496	2	2.205	2	0	1	0	1	0	1	0	3
284		min	-4803.291	3	-.1	3	0	1	0	1	0	1	-.006	2
285	10	max	3353.97	2	2.176	2	0	1	0	1	0	1	0	3
286		min	-4802.935	3	-.122	3	0	1	0	1	0	1	-.007	2
287	11	max	3354.443	2	2.147	2	0	1	0	1	0	1	0	3
288		min	-4802.58	3	-.143	3	0	1	0	1	0	1	-.007	2
289	12	max	3354.917	2	2.118	2	0	1	0	1	0	1	0	3
290		min	-4802.225	3	-.165	3	0	1	0	1	0	1	-.008	2
291	13	max	3355.391	2	2.089	2	0	1	0	1	0	1	0	3
292		min	-4801.869	3	-.187	3	0	1	0	1	0	1	-.009	2
293	14	max	3355.865	2	2.061	2	0	1	0	1	0	1	0	3
294		min	-4801.514	3	-.208	3	0	1	0	1	0	1	-.009	2
295	15	max	3356.338	2	2.032	2	0	1	0	1	0	1	0	3
296		min	-4801.159	3	-.23	3	0	1	0	1	0	1	-.01	2
297	16	max	3356.812	2	2.003	2	0	1	0	1	0	1	0	3
298		min	-4800.804	3	-.252	3	0	1	0	1	0	1	-.011	2
299	17	max	3357.286	2	1.974	2	0	1	0	1	0	1	0	3
300		min	-4800.448	3	-.273	3	0	1	0	1	0	1	-.011	2
301	18	max	3357.76	2	1.945	2	0	1	0	1	0	1	0	3
302		min	-4800.093	3	-.295	3	0	1	0	1	0	1	-.012	2
303	19	max	3358.233	2	1.916	2	0	1	0	1	0	1	0	3
304		min	-4799.738	3	-.316	3	0	1	0	1	0	1	-.013	2
305	M7	1	max	2449.35	2	9.017	4	0	1	0	0	1	.013	2
306		min	-2637.419	3	2.118	15	0	1	0	1	0	1	0	3
307	2	max	2449.179	2	8.145	4	0	1	0	1	0	1	.009	2
308		min	-2637.547	3	1.913	15	0	1	0	1	0	1	-.003	3
309	3	max	2449.009	2	7.273	4	0	1	0	1	0	1	.006	2
310		min	-2637.675	3	1.708	15	0	1	0	1	0	1	-.004	3
311	4	max	2448.839	2	6.401	4	0	1	0	1	0	1	.003	2



RISA-3D Version 13.0.0 \.....\PVM\Max 72 Cell 2V 25° 160mph 30psf 6.75ft 7-10Pa Page 25



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2832.529	2	0	1	0	1	0	1	0	1	0	1
370			min	-557.213	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2832.699	2	0	1	0	1	0	1	0	1	0	1
372			min	-557.085	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2832.87	2	0	1	0	1	0	1	0	1	0	1
374			min	-556.957	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2833.04	2	0	1	0	1	0	1	0	1	0	1
376			min	-556.83	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2833.21	2	0	1	0	1	0	1	0	1	0	1
378			min	-556.702	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2833.381	2	0	1	0	1	0	1	0	1	0	1
380			min	-556.574	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1135.679	2	2.025	4	-.014	15	0	1	0	2	0	1
382			min	-1578.237	3	.476	15	-.342	1	0	3	0	3	0	1
383		2	max	1136.153	2	1.988	4	-.014	15	0	1	0	10	0	15
384			min	-1577.881	3	.467	15	-.342	1	0	3	0	1	0	4
385		3	max	1136.627	2	1.951	4	-.014	15	0	1	0	15	0	15
386			min	-1577.526	3	.459	15	-.342	1	0	3	0	1	-.001	4
387		4	max	1137.1	2	1.914	4	-.014	15	0	1	0	15	0	15
388			min	-1577.171	3	.45	15	-.342	1	0	3	0	1	-.002	4
389		5	max	1137.574	2	1.877	4	-.014	15	0	1	0	15	0	15
390			min	-1576.815	3	.441	15	-.342	1	0	3	0	1	-.002	4
391		6	max	1138.048	2	1.84	4	-.014	15	0	1	0	15	0	15
392			min	-1576.46	3	.433	15	-.342	1	0	3	0	1	-.003	4
393		7	max	1138.522	2	1.803	4	-.014	15	0	1	0	15	0	15
394			min	-1576.105	3	.424	15	-.342	1	0	3	0	1	-.004	4
395		8	max	1138.995	2	1.766	4	-.014	15	0	1	0	15	0	15
396			min	-1575.749	3	.415	15	-.342	1	0	3	0	1	-.004	4
397		9	max	1139.469	2	1.729	4	-.014	15	0	1	0	15	-.001	15
398			min	-1575.394	3	.406	15	-.342	1	0	3	0	1	-.005	4
399		10	max	1139.943	2	1.692	4	-.014	15	0	1	0	15	-.001	15
400			min	-1575.039	3	.398	15	-.342	1	0	3	0	1	-.005	4
401		11	max	1140.417	2	1.655	4	-.014	15	0	1	0	15	-.001	15
402			min	-1574.684	3	.389	15	-.342	1	0	3	-.001	1	-.006	4
403		12	max	1140.89	2	1.618	4	-.014	15	0	1	0	15	-.002	15
404			min	-1574.328	3	.38	15	-.342	1	0	3	-.001	1	-.006	4
405		13	max	1141.364	2	1.581	4	-.014	15	0	1	0	15	-.002	15
406			min	-1573.973	3	.372	15	-.342	1	0	3	-.001	1	-.007	4
407		14	max	1141.838	2	1.544	4	-.014	15	0	1	0	15	-.002	15
408			min	-1573.618	3	.363	15	-.342	1	0	3	-.001	1	-.007	4
409		15	max	1142.312	2	1.507	4	-.014	15	0	1	0	15	-.002	15
410			min	-1573.262	3	.354	15	-.342	1	0	3	-.002	1	-.008	4
411		16	max	1142.785	2	1.47	4	-.014	15	0	1	0	15	-.002	15
412			min	-1572.907	3	.346	15	-.342	1	0	3	-.002	1	-.008	4
413		17	max	1143.259	2	1.433	4	-.014	15	0	1	0	15	-.002	15
414			min	-1572.552	3	.332	12	-.342	1	0	3	-.002	1	-.009	4
415		18	max	1143.733	2	1.396	4	-.014	15	0	1	0	15	-.002	15
416			min	-1572.196	3	.318	12	-.342	1	0	3	-.002	1	-.009	4
417		19	max	1144.207	2	1.359	4	-.014	15	0	1	0	15	-.002	15
418			min	-1571.841	3	.303	12	-.342	1	0	3	-.002	1	-.01	4
419	M11	1	max	773.217	2	8.995	4	-.007	15	0	1	0	15	.01	4
420			min	-910.32	3	2.114	15	-.164	1	0	5	0	1	.002	15
421		2	max	773.047	2	8.122	4	-.007	15	0	1	0	15	.006	2
422			min	-910.447	3	1.909	15	-.164	1	0	5	0	1	0	12
423		3	max	772.876	2	7.25	4	-.007	15	0	1	0	15	.003	2
424			min	-910.575	3	1.704	15	-.164	1	0	5	0	1	0	3
425		4	max	772.706	2	6.378	4	-.007	15	0	1	0	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-910.703	3	1.499	15	-.164	1	0	5	0	1	-.002	3
427		5	max	772.536	2	5.506	4	-.007	15	0	1	0	15	0	15
428			min	-910.831	3	1.294	15	-.164	1	0	5	0	1	-.004	3
429		6	max	772.365	2	4.634	4	-.007	15	0	1	0	15	-.001	15
430			min	-910.958	3	1.089	15	-.164	1	0	5	0	1	-.006	4
431		7	max	772.195	2	3.762	4	-.007	15	0	1	0	15	-.002	15
432			min	-911.086	3	.884	15	-.164	1	0	5	0	1	-.008	4
433		8	max	772.025	2	2.89	4	-.007	15	0	1	0	15	-.002	15
434			min	-911.214	3	.679	15	-.164	1	0	5	0	1	-.01	4
435		9	max	771.854	2	2.018	4	-.007	15	0	1	0	15	-.003	15
436			min	-911.342	3	.474	15	-.164	1	0	5	0	1	-.011	4
437		10	max	771.684	2	1.146	4	-.007	15	0	1	0	15	-.003	15
438			min	-911.469	3	.27	15	-.164	1	0	5	0	1	-.012	4
439		11	max	771.514	2	.4	2	-.007	15	0	1	0	15	-.003	15
440			min	-911.597	3	-.095	3	-.164	1	0	5	0	1	-.012	4
441		12	max	771.343	2	-.14	15	-.007	15	0	1	0	15	-.003	15
442			min	-911.725	3	-.605	3	-.164	1	0	5	0	1	-.012	4
443		13	max	771.173	2	-.345	15	-.007	15	0	1	0	15	-.003	15
444			min	-911.853	3	-1.47	4	-.164	1	0	5	-.001	1	-.012	4
445		14	max	771.002	2	-.55	15	-.007	15	0	1	0	15	-.003	15
446			min	-911.98	3	-2.342	4	-.164	1	0	5	-.001	1	-.011	4
447		15	max	770.832	2	-.755	15	-.007	15	0	1	0	15	-.002	15
448			min	-912.108	3	-3.214	4	-.164	1	0	5	-.001	1	-.009	4
449		16	max	770.662	2	-.96	15	-.007	15	0	1	0	15	-.002	15
450			min	-912.236	3	-4.086	4	-.164	1	0	5	-.001	1	-.008	4
451		17	max	770.491	2	-1.165	15	-.007	15	0	1	0	15	-.001	15
452			min	-912.364	3	-4.958	4	-.164	1	0	5	-.001	1	-.006	4
453		18	max	770.321	2	-1.37	15	-.007	15	0	1	0	15	0	15
454			min	-912.492	3	-5.83	4	-.164	1	0	5	-.001	1	-.003	4
455		19	max	770.151	2	-1.575	15	-.007	15	0	1	0	15	0	1
456			min	-912.619	3	-6.702	4	-.164	1	0	5	-.002	1	0	1
457	M12	1	max	1006.381	1	0	1	7.525	1	0	1	0	15	0	1
458			min	-157.505	3	0	1	.312	15	0	1	-.001	1	0	1
459		2	max	1006.552	1	0	1	7.525	1	0	1	0	10	0	1
460			min	-157.377	3	0	1	.312	15	0	1	0	1	0	1
461		3	max	1006.722	1	0	1	7.525	1	0	1	0	1	0	1
462			min	-157.249	3	0	1	.312	15	0	1	0	15	0	1
463		4	max	1006.892	1	0	1	7.525	1	0	1	.002	1	0	1
464			min	-157.122	3	0	1	.312	15	0	1	0	15	0	1
465		5	max	1007.063	1	0	1	7.525	1	0	1	.002	1	0	1
466			min	-156.994	3	0	1	.312	15	0	1	0	15	0	1
467		6	max	1007.233	1	0	1	7.525	1	0	1	.003	1	0	1
468			min	-156.866	3	0	1	.312	15	0	1	0	15	0	1
469		7	max	1007.403	1	0	1	7.525	1	0	1	.004	1	0	1
470			min	-156.738	3	0	1	.312	15	0	1	0	15	0	1
471		8	max	1007.574	1	0	1	7.525	1	0	1	.005	1	0	1
472			min	-156.61	3	0	1	.312	15	0	1	0	15	0	1
473		9	max	1007.744	1	0	1	7.525	1	0	1	.006	1	0	1
474			min	-156.483	3	0	1	.312	15	0	1	0	15	0	1
475		10	max	1007.914	1	0	1	7.525	1	0	1	.007	1	0	1
476			min	-156.355	3	0	1	.312	15	0	1	0	15	0	1
477		11	max	1008.085	1	0	1	7.525	1	0	1	.008	1	0	1
478			min	-156.227	3	0	1	.312	15	0	1	0	15	0	1
479		12	max	1008.255	1	0	1	7.525	1	0	1	.008	1	0	1
480			min	-156.099	3	0	1	.312	15	0	1	0	15	0	1
481		13	max	1008.425	1	0	1	7.525	1	0	1	.009	1	0	1
482			min	-155.972	3	0	1	.312	15	0	1	0	15	0	1



Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1094.757	2	-1806.05	3	0	1	0	1	0	1	-2.177	3
541		5	max	1771.768	3	1508.63	2	0	1	0	1	0	1	.094	1
542			min	-1094.045	2	-1807.195	3	0	1	0	1	0	1	-1.055	3
543		6	max	1772.302	3	1507.103	2	0	1	0	1	0	1	.067	3
544			min	-1093.333	2	-1808.34	3	0	1	0	1	0	1	-.879	2
545		7	max	1772.836	3	1505.576	2	0	1	0	1	0	1	1.189	3
546			min	-1092.621	2	-1809.486	3	0	1	0	1	0	1	-1.814	2
547		8	max	1773.37	3	1504.05	2	0	1	0	1	0	1	2.313	3
548			min	-1091.909	2	-1810.631	3	0	1	0	1	0	1	-2.748	2
549		9	max	1782.965	3	170.189	2	0	1	0	1	0	1	2.671	3
550			min	-965.329	2	.459	15	0	1	0	1	0	1	-3.152	2
551		10	max	1783.499	3	168.662	2	0	1	0	1	0	1	2.571	3
552			min	-964.617	2	-.002	15	0	1	0	1	0	1	-3.257	2
553		11	max	1784.033	3	167.136	2	0	1	0	1	0	1	2.472	3
554			min	-963.905	2	-1.805	4	0	1	0	1	0	1	-3.361	2
555		12	max	1794.41	3	1134.902	3	0	1	0	1	0	1	2.156	3
556			min	-837.685	2	-1841.878	2	0	1	0	1	0	1	-3	2
557		13	max	1794.944	3	1133.757	3	0	1	0	1	0	1	1.452	3
558			min	-836.973	2	-1843.405	2	0	1	0	1	0	1	-1.856	2
559		14	max	1795.478	3	1132.612	3	0	1	0	1	0	1	.749	3
560			min	-836.261	2	-1844.932	2	0	1	0	1	0	1	-.712	2
561		15	max	1796.012	3	1131.467	3	0	1	0	1	0	1	.434	2
562			min	-835.549	2	-1846.459	2	0	1	0	1	0	1	0	15
563		16	max	1796.546	3	1130.322	3	0	1	0	1	0	1	1.58	2
564			min	-834.837	2	-1847.986	2	0	1	0	1	0	1	-.656	3
565		17	max	1797.08	3	1129.177	3	0	1	0	1	0	1	2.727	2
566			min	-834.125	2	-1849.513	2	0	1	0	1	0	1	-1.357	3
567		18	max	-9.193	12	2225.506	2	0	1	0	1	0	1	1.39	2
568			min	-310.179	1	-1092.663	3	0	1	0	1	0	1	-.703	3
569		19	max	-8.837	12	2223.979	2	0	1	0	1	0	1	.011	1
570			min	-309.467	1	-1093.809	3	0	1	0	1	0	1	-.025	3
571	M9	1	max	135.271	1	830.488	3	43.245	1	0	3	-.005	15	0	15
572			min	5.517	15	-447.718	2	1.817	15	0	2	-.12	1	-.012	2
573		2	max	135.983	1	829.342	3	43.245	1	0	3	-.004	15	.266	2
574			min	5.732	15	-449.245	2	1.817	15	0	2	-.094	1	-.525	3
575		3	max	593.004	3	599.415	2	43.006	1	0	2	-.003	15	.534	2
576			min	-361.328	2	-646.778	3	1.803	15	0	3	-.067	1	-1.023	3
577		4	max	593.538	3	597.888	2	43.006	1	0	2	-.002	15	.162	2
578			min	-360.616	2	-647.923	3	1.803	15	0	3	-.04	1	-.621	3
579		5	max	594.072	3	596.361	2	43.006	1	0	2	0	15	-.005	15
580			min	-359.904	2	-649.068	3	1.803	15	0	3	-.013	1	-.219	3
581		6	max	594.606	3	594.834	2	43.006	1	0	2	.013	1	.184	3
582			min	-359.192	2	-650.213	3	1.803	15	0	3	0	15	-.578	2
583		7	max	595.14	3	593.307	2	43.006	1	0	2	.04	1	.588	3
584			min	-358.48	2	-651.359	3	1.803	15	0	3	.002	15	-.947	2
585		8	max	595.674	3	591.78	2	43.006	1	0	2	.067	1	.993	3
586			min	-357.768	2	-652.504	3	1.803	15	0	3	.003	15	-1.314	2
587		9	max	609.542	3	49.67	2	73.301	1	0	3	-.002	15	1.156	3
588			min	-302.446	2	.465	15	3.065	15	0	9	-.045	1	-1.497	2
589		10	max	610.076	3	48.143	2	73.301	1	0	3	0	1	1.132	3
590			min	-301.734	2	.005	15	3.065	15	0	9	0	10	-1.527	2
591		11	max	610.61	3	46.616	2	73.301	1	0	3	.046	1	1.108	3
592			min	-301.022	2	-1.876	4	3.065	15	0	9	.002	15	-1.557	2
593		12	max	624.087	3	431.843	3	41.766	1	0	3	-.003	15	.974	3
594			min	-245.521	2	-698.157	2	1.737	15	0	2	-.066	1	-1.383	2
595		13	max	624.621	3	430.698	3	41.766	1	0	3	-.002	15	.707	3
596			min	-244.809	2	-699.684	2	1.737	15	0	2	-.04	1	-.949	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	625.155	3	429.553	3	41.766	1	0	3	0	15	.44	3
598		min	-244.097	2	-701.211	2	1.737	15	0	2	-.014	1	-.514	2
599	15	max	625.689	3	428.407	3	41.766	1	0	3	.012	1	.173	3
600		min	-243.385	2	-702.738	2	1.737	15	0	2	0	15	-.094	1
601	16	max	626.223	3	427.262	3	41.766	1	0	3	.038	1	.358	2
602		min	-242.673	2	-704.265	2	1.737	15	0	2	.002	15	-.092	3
603	17	max	626.757	3	426.117	3	41.766	1	0	3	.064	1	.796	2
604		min	-241.961	2	-705.792	2	1.737	15	0	2	.003	15	-.357	3
605	18	max	-5.746	15	656.917	2	48.942	1	0	2	.093	1	.402	2
606		min	-136.661	1	-304.065	3	2.042	15	0	3	.004	15	-.177	3
607	19	max	-5.531	15	655.39	2	48.942	1	0	2	.123	1	.012	3
608		min	-135.949	1	-305.21	3	2.042	15	0	3	.005	15	-.006	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.237	2	.011	3	1.626e-2	2	NC	1	NC	1
2			min	0	15	-.071	3	-.007	2	-4.686e-3	3	NC	1	NC	1
3		2	max	0	1	.188	2	.014	3	1.706e-2	2	NC	4	NC	1
4			min	0	15	.004	15	-.004	10	-4.044e-3	3	1218.868	3	NC	1
5		3	max	0	1	.172	3	.024	1	1.787e-2	2	NC	4	NC	2
6			min	0	15	.003	15	-.003	10	-3.403e-3	3	667.325	3	6359.385	1
7		4	max	0	1	.243	3	.035	1	1.868e-2	2	NC	5	NC	2
8			min	0	15	.003	15	-.003	10	-2.761e-3	3	516.009	3	4398.44	1
9		5	max	0	1	.268	3	.04	1	1.948e-2	2	NC	5	NC	2
10			min	0	15	.003	15	-.004	10	-2.119e-3	3	477.87	3	3889.173	1
11		6	max	0	1	.248	3	.037	1	2.029e-2	2	NC	4	NC	2
12			min	0	15	.004	15	-.006	10	-1.478e-3	3	508.478	3	4213.501	1
13		7	max	0	1	.222	2	.03	3	2.11e-2	2	NC	2	NC	2
14			min	0	15	.004	15	-.008	10	-8.364e-4	3	619.43	3	5808.498	1
15		8	max	0	1	.278	2	.032	3	2.191e-2	2	NC	4	NC	1
16			min	0	15	.006	15	-.013	2	-1.949e-4	3	876.904	3	7914.405	3
17		9	max	0	1	.326	2	.033	3	2.271e-2	2	NC	4	NC	1
18			min	0	15	.007	15	-.02	2	4.457e-4	15	1429.821	3	7569.166	3
19		10	max	0	1	.347	2	.033	3	2.352e-2	2	NC	4	NC	1
20		min	0	1	.007	15	-.023	2	4.563e-4	15	1478.124	2	7477.568	3	
21	11	max	0	15	.326	2	.033	3	2.271e-2	2	NC	4	NC	1	
22		min	0	1	.007	15	-.02	2	4.457e-4	15	1429.821	3	7569.166	3	
23	12	max	0	15	.278	2	.032	3	2.191e-2	2	NC	4	NC	1	
24		min	0	1	.006	15	-.013	2	-1.949e-4	3	876.904	3	7914.405	3	
25	13	max	0	15	.222	2	.03	3	2.11e-2	2	NC	2	NC	2	
26		min	0	1	.004	15	-.008	10	-8.364e-4	3	619.43	3	5808.498	1	
27	14	max	0	15	.248	3	.037	1	2.029e-2	2	NC	4	NC	2	
28		min	0	1	.004	15	-.006	10	-1.478e-3	3	508.478	3	4213.501	1	
29	15	max	0	15	.268	3	.04	1	1.948e-2	2	NC	5	NC	2	
30		min	0	1	.003	15	-.004	10	-2.119e-3	3	477.87	3	3889.173	1	
31	16	max	0	15	.243	3	.035	1	1.868e-2	2	NC	5	NC	2	
32		min	0	1	.003	15	-.003	10	-2.761e-3	3	516.009	3	4398.44	1	
33	17	max	0	15	.172	3	.024	1	1.787e-2	2	NC	4	NC	2	
34		min	0	1	.003	15	-.003	10	-3.403e-3	3	667.325	3	6359.385	1	
35	18	max	0	15	.188	2	.014	3	1.706e-2	2	NC	4	NC	1	
36		min	0	1	.004	15	-.004	10	-4.044e-3	3	1218.868	3	NC	1	
37	19	max	0	15	.237	2	.011	3	1.626e-2	2	NC	1	NC	1	
38		min	0	1	-.071	3	-.007	2	-4.686e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.486	3	.01	3	8.891e-3	2	NC	1	NC	1
40			min	0	15	-.696	2	-.006	2	-7.287e-3	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.666	3	.011	3	1.001e-2	2	NC	5	NC	1
42		min	0	15	-.878	2	-.004	2	-8.327e-3	3	890.453	2	NC	1
43	3	max	0	1	.827	3	.018	1	1.112e-2	2	NC	5	NC	2
44		min	0	15	-1.044	2	-.003	10	-9.366e-3	3	465.215	2	8653.375	1
45	4	max	0	1	.954	3	.028	1	1.224e-2	2	NC	5	NC	2
46		min	0	15	-1.184	2	-.003	10	-1.04e-2	3	332.34	2	5490.043	1
47	5	max	0	1	1.041	3	.034	1	1.335e-2	2	NC	15	NC	2
48		min	0	15	-1.289	2	-.004	10	-1.144e-2	3	273.301	2	4622.091	1
49	6	max	0	1	1.086	3	.032	1	1.447e-2	2	NC	15	NC	2
50		min	0	15	-1.358	2	-.005	10	-1.248e-2	3	244.767	2	4848.484	1
51	7	max	0	1	1.093	3	.026	3	1.558e-2	2	NC	15	NC	2
52		min	0	15	-1.393	2	-.007	10	-1.352e-2	3	232.406	2	6521.992	1
53	8	max	0	1	1.073	3	.028	3	1.67e-2	2	NC	15	NC	1
54		min	0	15	-1.401	2	-.012	2	-1.456e-2	3	229.708	2	9041.854	3
55	9	max	0	1	1.043	3	.029	3	1.781e-2	2	NC	15	NC	1
56		min	0	15	-1.394	2	-.018	2	-1.56e-2	3	232.132	2	8578.236	3
57	10	max	0	1	1.027	3	.029	3	1.893e-2	2	NC	15	NC	1
58		min	0	1	-1.387	2	-.021	2	-1.664e-2	3	234.439	2	8451.634	3
59	11	max	0	15	1.043	3	.029	3	1.781e-2	2	NC	15	NC	1
60		min	0	1	-1.394	2	-.018	2	-1.56e-2	3	232.132	2	8578.236	3
61	12	max	0	15	1.073	3	.028	3	1.67e-2	2	NC	15	NC	1
62		min	0	1	-1.401	2	-.012	2	-1.456e-2	3	229.708	2	9041.854	3
63	13	max	0	15	1.093	3	.026	3	1.558e-2	2	NC	15	NC	2
64		min	0	1	-1.393	2	-.007	10	-1.352e-2	3	232.406	2	6521.992	1
65	14	max	0	15	1.086	3	.032	1	1.447e-2	2	NC	15	NC	2
66		min	0	1	-1.358	2	-.005	10	-1.248e-2	3	244.767	2	4848.484	1
67	15	max	0	15	1.041	3	.034	1	1.335e-2	2	NC	15	NC	2
68		min	0	1	-1.289	2	-.004	10	-1.144e-2	3	273.301	2	4622.091	1
69	16	max	0	15	.954	3	.028	1	1.224e-2	2	NC	5	NC	2
70		min	0	1	-1.184	2	-.003	10	-1.04e-2	3	332.34	2	5490.043	1
71	17	max	0	15	.827	3	.018	1	1.112e-2	2	NC	5	NC	2
72		min	0	1	-1.044	2	-.003	10	-9.366e-3	3	465.215	2	8653.375	1
73	18	max	0	15	.666	3	.011	3	1.001e-2	2	NC	5	NC	1
74		min	0	1	-.878	2	-.004	2	-8.327e-3	3	890.453	2	NC	1
75	19	max	0	15	.486	3	.01	3	8.891e-3	2	NC	1	NC	1
76		min	0	1	-.696	2	-.006	2	-7.287e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.498	.009	3	6.148e-3	3	NC	1	NC	1
78		min	0	1	-.695	2	-.006	2	-9.227e-3	2	NC	1	NC	1
79	2	max	0	15	.636	3	.011	3	7.006e-3	3	NC	5	NC	1
80		min	0	1	-.906	2	-.004	2	-1.039e-2	2	766.89	2	NC	1
81	3	max	0	15	.764	3	.018	1	7.864e-3	3	NC	5	NC	2
82		min	0	1	-1.096	2	-.003	10	-1.156e-2	2	403.689	2	8583.449	1
83	4	max	0	15	.872	3	.029	1	8.722e-3	3	NC	5	NC	2
84		min	0	1	-1.25	2	-.003	10	-1.272e-2	2	291.893	2	5448.218	1
85	5	max	0	15	.955	3	.034	1	9.58e-3	3	NC	15	NC	2
86		min	0	1	-1.358	2	-.003	10	-1.389e-2	2	244.071	2	4583.406	1
87	6	max	0	15	1.013	3	.033	1	1.044e-2	3	NC	15	NC	2
88		min	0	1	-1.42	2	-.005	10	-1.505e-2	2	223.297	2	4797.092	1
89	7	max	0	15	1.045	3	.025	3	1.13e-2	3	NC	15	NC	2
90		min	0	1	-1.439	2	-.007	10	-1.622e-2	2	217.513	2	6415.953	1
91	8	max	0	15	1.057	3	.026	3	1.215e-2	3	NC	15	NC	1
92		min	0	1	-1.427	2	-.011	2	-1.738e-2	2	221.098	2	9733.038	3
93	9	max	0	15	1.055	3	.027	3	1.301e-2	3	NC	15	NC	1
94		min	0	1	-1.402	2	-.017	2	-1.855e-2	2	229.158	2	9273.31	3
95	10	max	0	1	1.052	3	.027	3	1.387e-2	3	NC	15	NC	1
96		min	0	1	-1.386	2	-.02	2	-1.971e-2	2	234.208	2	9153.434	3
97	11	max	0	1	1.055	3	.027	3	1.301e-2	3	NC	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.402	2	-.017	2	-1.855e-2	2	229.158	2	9273.31	3
99		12	max	0	1	1.057	3	.026	3	1.215e-2	3	NC	15	NC	1
100			min	0	15	-1.427	2	-.011	2	-1.738e-2	2	221.098	2	9733.038	3
101		13	max	0	1	1.045	3	.025	3	1.13e-2	3	NC	15	NC	2
102			min	0	15	-1.439	2	-.007	10	-1.622e-2	2	217.513	2	6415.953	1
103		14	max	0	1	1.013	3	.033	1	1.044e-2	3	NC	15	NC	2
104			min	0	15	-1.42	2	-.005	10	-1.505e-2	2	223.297	2	4797.092	1
105		15	max	0	1	.955	3	.034	1	9.58e-3	3	NC	15	NC	2
106			min	0	15	-1.358	2	-.003	10	-1.389e-2	2	244.071	2	4583.406	1
107		16	max	0	1	.872	3	.029	1	8.722e-3	3	NC	5	NC	2
108			min	0	15	-1.25	2	-.003	10	-1.272e-2	2	291.893	2	5448.218	1
109		17	max	0	1	.764	3	.018	1	7.864e-3	3	NC	5	NC	2
110			min	0	15	-1.096	2	-.003	10	-1.156e-2	2	403.689	2	8583.449	1
111		18	max	0	1	.636	3	.011	3	7.006e-3	3	NC	5	NC	1
112			min	0	15	-.906	2	-.004	2	-1.039e-2	2	766.89	2	NC	1
113		19	max	0	1	.498	3	.009	3	6.148e-3	3	NC	1	NC	1
114			min	0	15	-.695	2	-.006	2	-9.227e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.213	2	.008	3	1.206e-2	3	NC	1	NC	1
116			min	0	1	-.179	3	-.006	2	-1.391e-2	2	NC	1	NC	1
117		2	max	0	15	.127	2	.01	1	1.289e-2	3	NC	4	NC	1
118			min	0	1	-.153	3	-.003	10	-1.422e-2	2	1885.629	2	NC	1
119		3	max	0	15	.074	1	.025	1	1.371e-2	3	NC	4	NC	2
120			min	0	1	-.134	3	-.002	10	-1.454e-2	2	1053.715	2	6337.77	1
121		4	max	0	15	.047	1	.036	1	1.453e-2	3	NC	4	NC	2
122			min	0	1	-.129	3	-.002	10	-1.486e-2	2	846.302	2	4356.824	1
123		5	max	0	15	.048	1	.041	1	1.535e-2	3	NC	4	NC	2
124			min	0	1	-.141	3	-.002	10	-1.518e-2	2	837.417	2	3824.638	1
125		6	max	0	15	.076	1	.039	1	1.617e-2	3	NC	3	NC	2
126			min	0	1	-.168	3	-.003	10	-1.549e-2	2	1006.82	2	4096.945	1
127		7	max	0	15	.125	1	.028	1	1.699e-2	3	NC	4	NC	2
128			min	0	1	-.205	3	-.005	10	-1.581e-2	2	1597.522	2	5516.071	1
129		8	max	0	15	.184	2	.023	3	1.781e-2	3	NC	1	NC	1
130			min	0	1	-.247	3	-.008	2	-1.613e-2	2	2384.596	3	NC	1
131		9	max	0	15	.248	2	.023	3	1.863e-2	3	NC	4	NC	1
132			min	0	1	-.283	3	-.015	2	-1.645e-2	2	1566.643	3	NC	1
133		10	max	0	1	.277	2	.023	3	1.946e-2	3	NC	4	NC	1
134			min	0	1	-.298	3	-.018	2	-1.676e-2	2	1362.244	3	NC	1
135		11	max	0	1	.248	2	.023	3	1.863e-2	3	NC	4	NC	1
136			min	0	15	-.283	3	-.015	2	-1.645e-2	2	1566.643	3	NC	1
137		12	max	0	1	.184	2	.023	3	1.781e-2	3	NC	1	NC	1
138			min	0	15	-.247	3	-.008	2	-1.613e-2	2	2384.596	3	NC	1
139		13	max	0	1	.125	1	.028	1	1.699e-2	3	NC	4	NC	2
140			min	0	15	-.205	3	-.005	10	-1.581e-2	2	1597.522	2	5516.071	1
141		14	max	0	1	.076	1	.039	1	1.617e-2	3	NC	3	NC	2
142			min	0	15	-.168	3	-.003	10	-1.549e-2	2	1006.82	2	4096.945	1
143		15	max	0	1	.048	1	.041	1	1.535e-2	3	NC	4	NC	2
144			min	0	15	-.141	3	-.002	10	-1.518e-2	2	837.417	2	3824.638	1
145		16	max	0	1	.047	1	.036	1	1.453e-2	3	NC	4	NC	2
146			min	0	15	-.129	3	-.002	10	-1.486e-2	2	846.302	2	4356.824	1
147		17	max	0	1	.074	1	.025	1	1.371e-2	3	NC	4	NC	2
148			min	0	15	-.134	3	-.002	10	-1.454e-2	2	1053.715	2	6337.77	1
149		18	max	0	1	.127	2	.01	1	1.289e-2	3	NC	4	NC	1
150			min	0	15	-.153	3	-.003	10	-1.422e-2	2	1885.629	2	NC	1
151		19	max	0	1	.213	2	.008	3	1.206e-2	3	NC	1	NC	1
152			min	0	15	-.179	3	-.006	2	-1.391e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.011	2	.006	1	-5.095e-6	15	NC	1	NC	1
154			min	-.01	3	-.016	3	0	15	-1.21e-4	1	6432.269	2	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.007	2	.009	2	.005	1	-4.815e-6	15	NC	1	NC	1
156		min	-.01	3	-.016	3	0	15	-1.143e-4	1	7374.517	2	NC	1
157	3	max	.007	2	.008	2	.005	1	-4.535e-6	15	NC	1	NC	1
158		min	-.009	3	-.015	3	0	15	-1.077e-4	1	8623.638	2	NC	1
159	4	max	.006	2	.007	2	.004	1	-4.254e-6	15	NC	1	NC	1
160		min	-.009	3	-.015	3	0	15	-1.01e-4	1	NC	1	NC	1
161	5	max	.006	2	.005	2	.004	1	-3.974e-6	15	NC	1	NC	1
162		min	-.008	3	-.014	3	0	15	-9.432e-5	1	NC	1	NC	1
163	6	max	.005	2	.004	2	.003	1	-3.694e-6	15	NC	1	NC	1
164		min	-.008	3	-.013	3	0	15	-8.764e-5	1	NC	1	NC	1
165	7	max	.005	2	.003	2	.003	1	-3.414e-6	15	NC	1	NC	1
166		min	-.007	3	-.013	3	0	15	-8.097e-5	1	NC	1	NC	1
167	8	max	.005	2	.002	2	.003	1	-3.133e-6	15	NC	1	NC	1
168		min	-.006	3	-.012	3	0	15	-7.429e-5	1	NC	1	NC	1
169	9	max	.004	2	.001	2	.002	1	-2.853e-6	15	NC	1	NC	1
170		min	-.006	3	-.011	3	0	15	-6.762e-5	1	NC	1	NC	1
171	10	max	.004	2	0	2	.002	1	-2.573e-6	15	NC	1	NC	1
172		min	-.005	3	-.01	3	0	15	-6.094e-5	1	NC	1	NC	1
173	11	max	.003	2	0	2	.001	1	-2.292e-6	15	NC	1	NC	1
174		min	-.005	3	-.009	3	0	15	-5.427e-5	1	NC	1	NC	1
175	12	max	.003	2	-.001	2	.001	1	-2.012e-6	15	NC	1	NC	1
176		min	-.004	3	-.009	3	0	15	-4.759e-5	1	NC	1	NC	1
177	13	max	.003	2	-.001	15	0	1	-1.732e-6	15	NC	1	NC	1
178		min	-.003	3	-.008	3	0	15	-4.092e-5	1	NC	1	NC	1
179	14	max	.002	2	-.001	15	0	1	-1.452e-6	15	NC	1	NC	1
180		min	-.003	3	-.006	3	0	15	-3.424e-5	1	NC	1	NC	1
181	15	max	.002	2	-.001	15	0	1	-1.171e-6	15	NC	1	NC	1
182		min	-.002	3	-.005	3	0	15	-2.757e-5	1	NC	1	NC	1
183	16	max	.001	2	0	15	0	1	-8.911e-7	15	NC	1	NC	1
184		min	-.002	3	-.004	3	0	15	-2.089e-5	1	NC	1	NC	1
185	17	max	0	2	0	15	0	1	-6.108e-7	15	NC	1	NC	1
186		min	-.001	3	-.003	3	0	15	-1.421e-5	1	NC	1	NC	1
187	18	max	0	2	0	15	0	1	-2.398e-7	10	NC	1	NC	1
188		min	0	3	-.002	4	0	15	-7.539e-6	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.763e-7	10	NC	1	NC	1
190		min	0	1	0	1	0	1	-1.031e-6	3	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1.036e-7	3	NC	1	NC	1
192		min	0	1	0	1	0	1	-6.148e-7	1	NC	1	NC	1
193	2	max	0	3	0	15	0	2	1.47e-5	1	NC	1	NC	1
194		min	0	2	-.003	4	0	3	6.121e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	3.002e-5	1	NC	1	NC	1
196		min	0	2	-.006	4	0	3	1.247e-6	15	NC	1	NC	1
197	4	max	.001	3	-.002	15	0	1	4.534e-5	1	NC	1	NC	1
198		min	-.001	2	-.009	4	0	3	1.881e-6	15	NC	1	NC	1
199	5	max	.002	3	-.003	15	0	1	6.066e-5	1	NC	1	NC	1
200		min	-.002	2	-.012	4	0	3	2.516e-6	15	8805.403	4	NC	1
201	6	max	.002	3	-.003	15	0	1	7.598e-5	1	NC	2	NC	1
202		min	-.002	2	-.015	4	0	3	3.15e-6	15	7107.543	4	NC	1
203	7	max	.003	3	-.004	15	0	1	9.13e-5	1	NC	5	NC	1
204		min	-.003	2	-.017	4	0	12	3.785e-6	15	6086.379	4	NC	1
205	8	max	.003	3	-.004	15	0	1	1.066e-4	1	NC	5	NC	1
206		min	-.003	2	-.019	4	0	15	4.419e-6	15	5456.22	4	NC	1
207	9	max	.004	3	-.005	15	0	1	1.219e-4	1	NC	5	NC	1
208		min	-.003	2	-.02	4	0	15	5.054e-6	15	5082.698	4	NC	1
209	10	max	.004	3	-.005	15	0	1	1.373e-4	1	NC	5	NC	1
210		min	-.004	2	-.021	4	0	15	5.688e-6	15	4900.5	4	NC	1
211	11	max	.005	3	-.005	15	.001	1	1.526e-4	1	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.021	4	0	15	6.323e-6	15	4882.701	4	NC	1
213		max	.005	3	-.005	15	.001	1	1.679e-4	1	NC	5	NC	1
214		min	-.005	2	-.021	4	0	15	6.957e-6	15	5030.428	4	NC	1
215		max	.006	3	-.005	15	.002	1	1.832e-4	1	NC	5	NC	1
216		min	-.005	2	-.019	4	0	15	7.592e-6	15	5374.608	4	NC	1
217		max	.006	3	-.004	15	.002	1	1.985e-4	1	NC	5	NC	1
218		min	-.005	2	-.017	4	0	15	8.226e-6	15	5992.09	4	NC	1
219		max	.007	3	-.003	15	.003	1	2.139e-4	1	NC	3	NC	1
220		min	-.006	2	-.015	4	0	15	8.861e-6	15	7053.558	4	NC	1
221		max	.007	3	-.003	15	.003	1	2.292e-4	1	NC	1	NC	1
222		min	-.006	2	-.012	4	0	15	9.495e-6	15	8972.668	4	NC	1
223		max	.008	3	-.002	15	.004	1	2.445e-4	1	NC	1	NC	1
224		min	-.007	2	-.008	4	0	15	1.013e-5	15	NC	1	NC	1
225		max	.008	3	-.001	15	.005	1	2.598e-4	1	NC	1	NC	1
226		min	-.007	2	-.005	1	0	15	1.076e-5	15	NC	1	NC	1
227		max	.009	3	0	10	.005	1	2.751e-4	1	NC	1	NC	1
228		min	-.008	2	-.002	3	0	15	1.14e-5	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	7.563e-5	1	NC	1	NC	2
230		min	0	3	-.009	3	-.005	1	3.161e-6	15	NC	1	4683.99	1
231		max	.002	1	.007	2	0	15	7.563e-5	1	NC	1	NC	2
232		min	0	3	-.009	3	-.005	1	3.161e-6	15	NC	1	5088.46	1
233		max	.002	1	.006	2	0	15	7.563e-5	1	NC	1	NC	2
234		min	0	3	-.008	3	-.004	1	3.161e-6	15	NC	1	5570.145	1
235		max	.002	1	.006	2	0	15	7.563e-5	1	NC	1	NC	2
236		min	0	3	-.008	3	-.004	1	3.161e-6	15	NC	1	6149.051	1
237		max	.002	1	.006	2	0	15	7.563e-5	1	NC	1	NC	2
238		min	0	3	-.007	3	-.004	1	3.161e-6	15	NC	1	6852.367	1
239		max	.002	1	.005	2	0	15	7.563e-5	1	NC	1	NC	2
240		min	0	3	-.007	3	-.003	1	3.161e-6	15	NC	1	7717.8	1
241		max	.002	1	.005	2	0	15	7.563e-5	1	NC	1	NC	2
242		min	0	3	-.006	3	-.003	1	3.161e-6	15	NC	1	8798.872	1
243		max	.001	1	.004	2	0	15	7.563e-5	1	NC	1	NC	1
244		min	0	3	-.006	3	-.002	1	3.161e-6	15	NC	1	NC	1
245		max	.001	1	.004	2	0	15	7.563e-5	1	NC	1	NC	1
246		min	0	3	-.005	3	-.002	1	3.161e-6	15	NC	1	NC	1
247		max	.001	1	.004	2	0	15	7.563e-5	1	NC	1	NC	1
248		min	0	3	-.005	3	-.002	1	3.161e-6	15	NC	1	NC	1
249		max	.001	1	.003	2	0	15	7.563e-5	1	NC	1	NC	1
250		min	0	3	-.004	3	-.001	1	3.161e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	7.563e-5	1	NC	1	NC	1
252		min	0	3	-.004	3	-.001	1	3.161e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	7.563e-5	1	NC	1	NC	1
254		min	0	3	-.003	3	0	1	3.161e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	7.563e-5	1	NC	1	NC	1
256		min	0	3	-.003	3	0	1	3.161e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	7.563e-5	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	3.161e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	7.563e-5	1	NC	1	NC	1
260		min	0	3	-.002	3	0	1	3.161e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	7.563e-5	1	NC	1	NC	1
262		min	0	3	-.001	3	0	1	3.161e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	7.563e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	3.161e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	7.563e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	3.161e-6	15	NC	1	NC	1
267	M6	max	.022	2	.034	2	0	1	0	1	NC	4	NC	1
268		min	-.032	3	-.048	3	0	1	0	1	1433.601	3	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.022	3	0	1	0	1	4933.05	4	NC	1
327		12	max	.016	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.015	2	-.021	3	0	1	0	1	5079.864	4	NC	1
329		13	max	.017	3	-.005	15	0	1	0	1	NC	5	NC	1
330			min	-.016	2	-.02	3	0	1	0	1	5425.245	4	NC	1
331		14	max	.019	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.017	2	-.019	3	0	1	0	1	6046.525	4	NC	1
333		15	max	.02	3	-.004	15	0	1	0	1	NC	1	NC	1
334			min	-.019	2	-.017	3	0	1	0	1	7115.7	4	NC	1
335		16	max	.022	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.02	2	-.014	3	0	1	0	1	9049.783	4	NC	1
337		17	max	.023	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.021	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.024	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.023	2	-.008	3	0	1	0	1	NC	1	NC	1
341		19	max	.026	3	0	10	0	1	0	1	NC	1	NC	1
342			min	-.024	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	2	.023	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.026	3	0	1	0	1	NC	1	NC	1
345		2	max	.006	2	.022	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.025	3	0	1	0	1	NC	1	NC	1
347		3	max	.006	2	.021	2	0	1	0	1	NC	1	NC	1
348			min	-.001	3	-.023	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	2	.019	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.022	3	0	1	0	1	NC	1	NC	1
351		5	max	.005	2	.018	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.021	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	2	.017	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.019	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	2	.015	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	2	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	2	.013	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	2	.012	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.012	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	2	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	2	.008	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	2	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	2	.011	2	0	15	1.21e-4	1	NC	1	NC	1
382			min	-.01	3	-.016	3	-.006	1	5.095e-6	15	6432.269	2	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.004	2	-.021	4	-.001	1	-1.526e-4	1	4882.701	4	NC	1
441		max	.005	3	-.005	15	0	15	-6.957e-6	15	NC	5	NC	1
442		min	-.005	2	-.021	4	-.001	1	-1.679e-4	1	5030.428	4	NC	1
443		max	.006	3	-.005	15	0	15	-7.592e-6	15	NC	5	NC	1
444		min	-.005	2	-.019	4	-.002	1	-1.832e-4	1	5374.608	4	NC	1
445		max	.006	3	-.004	15	0	15	-8.226e-6	15	NC	5	NC	1
446		min	-.005	2	-.017	4	-.002	1	-1.985e-4	1	5992.09	4	NC	1
447		max	.007	3	-.003	15	0	15	-8.861e-6	15	NC	3	NC	1
448		min	-.006	2	-.015	4	-.003	1	-2.139e-4	1	7053.558	4	NC	1
449		max	.007	3	-.003	15	0	15	-9.495e-6	15	NC	1	NC	1
450		min	-.006	2	-.012	4	-.003	1	-2.292e-4	1	8972.668	4	NC	1
451		max	.008	3	-.002	15	0	15	-1.013e-5	15	NC	1	NC	1
452		min	-.007	2	-.008	4	-.004	1	-2.445e-4	1	NC	1	NC	1
453		max	.008	3	-.001	15	0	15	-1.076e-5	15	NC	1	NC	1
454		min	-.007	2	-.005	1	-.005	1	-2.598e-4	1	NC	1	NC	1
455		max	.009	3	0	10	0	15	-1.14e-5	15	NC	1	NC	1
456		min	-.008	2	-.002	3	-.005	1	-2.751e-4	1	NC	1	NC	1
457	M12	max	.002	1	.007	2	.005	1	-3.161e-6	15	NC	1	NC	2
458		min	0	3	-.009	3	0	15	-7.563e-5	1	NC	1	4683.99	1
459		max	.002	1	.007	2	.005	1	-3.161e-6	15	NC	1	NC	2
460		min	0	3	-.009	3	0	15	-7.563e-5	1	NC	1	5088.46	1
461		max	.002	1	.006	2	.004	1	-3.161e-6	15	NC	1	NC	2
462		min	0	3	-.008	3	0	15	-7.563e-5	1	NC	1	5570.145	1
463		max	.002	1	.006	2	.004	1	-3.161e-6	15	NC	1	NC	2
464		min	0	3	-.008	3	0	15	-7.563e-5	1	NC	1	6149.051	1
465		max	.002	1	.006	2	.004	1	-3.161e-6	15	NC	1	NC	2
466		min	0	3	-.007	3	0	15	-7.563e-5	1	NC	1	6852.367	1
467		max	.002	1	.005	2	.003	1	-3.161e-6	15	NC	1	NC	2
468		min	0	3	-.007	3	0	15	-7.563e-5	1	NC	1	7717.8	1
469		max	.002	1	.005	2	.003	1	-3.161e-6	15	NC	1	NC	2
470		min	0	3	-.006	3	0	15	-7.563e-5	1	NC	1	8798.872	1
471		max	.001	1	.004	2	.002	1	-3.161e-6	15	NC	1	NC	1
472		min	0	3	-.006	3	0	15	-7.563e-5	1	NC	1	NC	1
473		max	.001	1	.004	2	.002	1	-3.161e-6	15	NC	1	NC	1
474		min	0	3	-.005	3	0	15	-7.563e-5	1	NC	1	NC	1
475		max	.001	1	.004	2	.002	1	-3.161e-6	15	NC	1	NC	1
476		min	0	3	-.005	3	0	15	-7.563e-5	1	NC	1	NC	1
477		max	.001	1	.003	2	.001	1	-3.161e-6	15	NC	1	NC	1
478		min	0	3	-.004	3	0	15	-7.563e-5	1	NC	1	NC	1
479		max	0	1	.003	2	.001	1	-3.161e-6	15	NC	1	NC	1
480		min	0	3	-.004	3	0	15	-7.563e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-3.161e-6	15	NC	1	NC	1
482		min	0	3	-.003	3	0	15	-7.563e-5	1	NC	1	NC	1
483		max	0	1	.002	2	0	1	-3.161e-6	15	NC	1	NC	1
484		min	0	3	-.003	3	0	15	-7.563e-5	1	NC	1	NC	1
485		max	0	1	.002	2	0	1	-3.161e-6	15	NC	1	NC	1
486		min	0	3	-.002	3	0	15	-7.563e-5	1	NC	1	NC	1
487		max	0	1	.001	2	0	1	-3.161e-6	15	NC	1	NC	1
488		min	0	3	-.002	3	0	15	-7.563e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-3.161e-6	15	NC	1	NC	1
490		min	0	3	-.001	3	0	15	-7.563e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-3.161e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-7.563e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-3.161e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-7.563e-5	1	NC	1	NC	1
495	M1	max	.011	3	.237	2	0	1	5.752e-3	2	NC	1	NC	1
496		min	-.007	2	-.071	3	0	15	-1.53e-2	3	NC	1	NC	1







Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

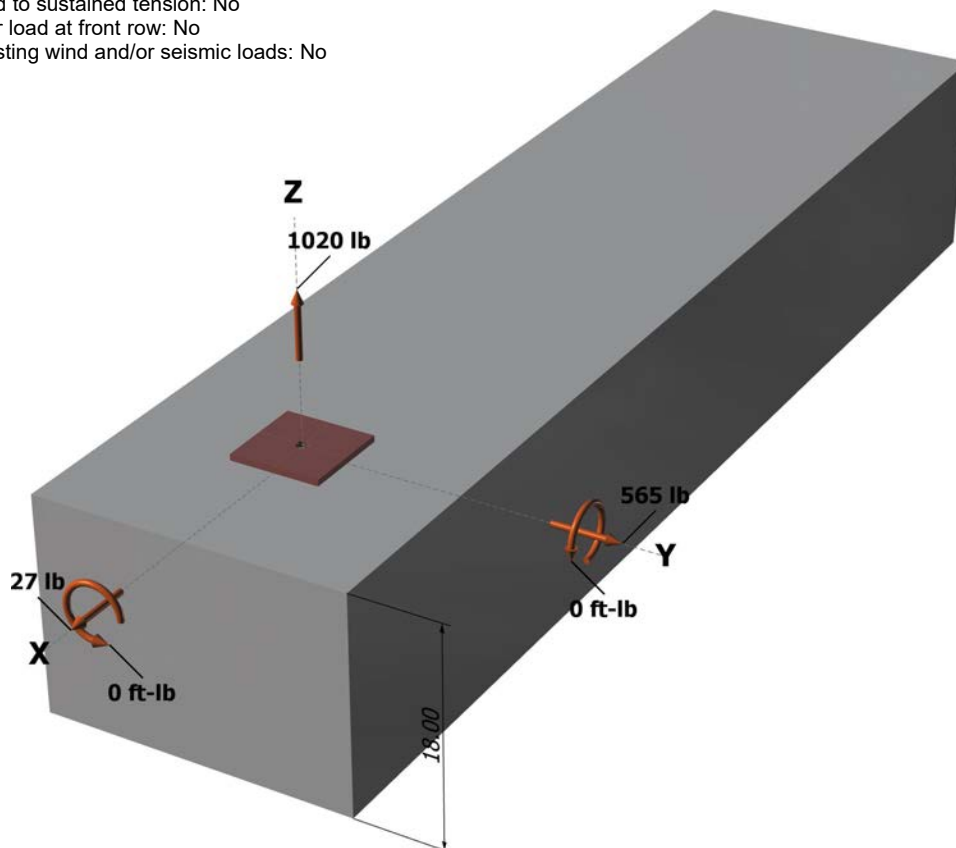
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

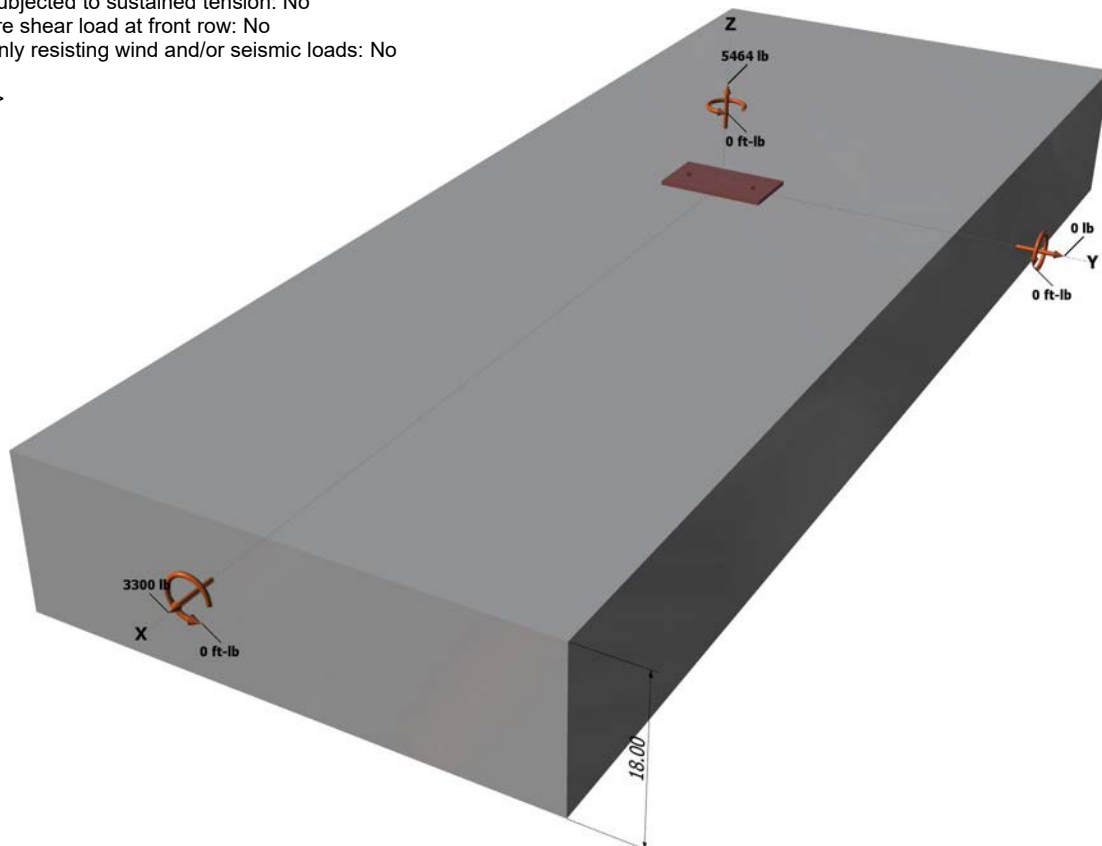
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

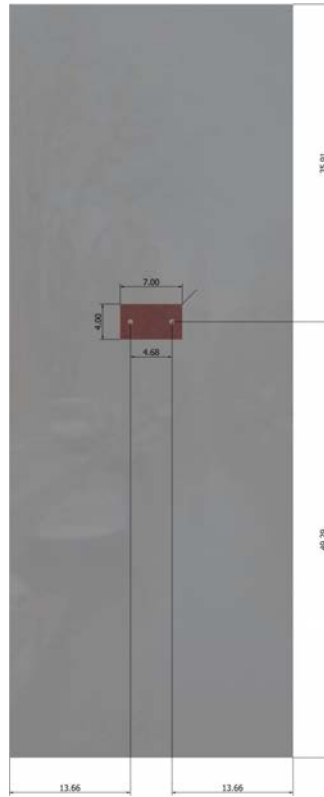
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.