

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.053 k-ft
$P_n$ =	0.269 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>15%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

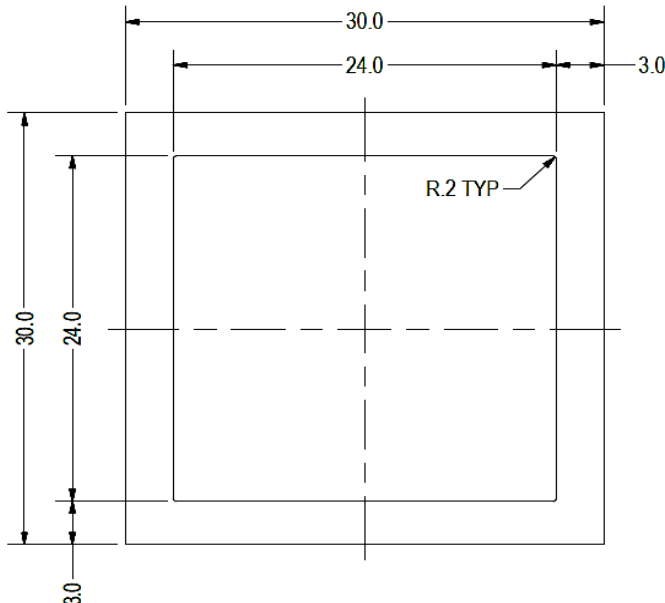
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.476 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>12%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

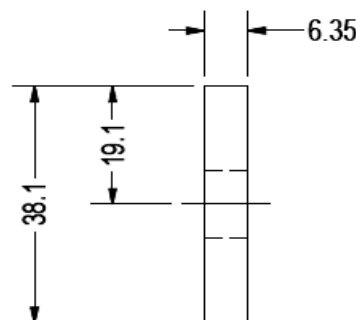
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.087 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<u>19%</u>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.007 k-ft
$P_n$ =	0.245 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>17%</u>



A cross brace kit is required every 11 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

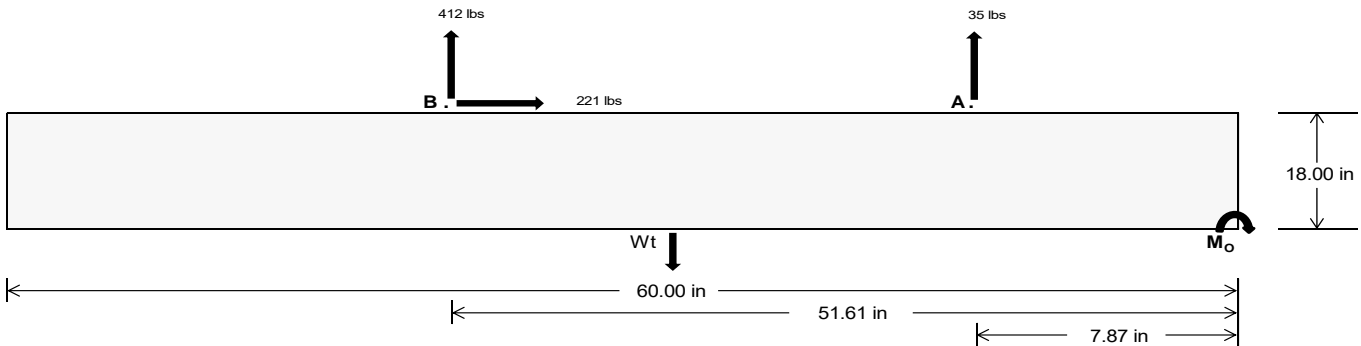
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>151.95</u>	<u>1716.66</u>	k
Compressive Load =	<u>1700.79</u>	<u>1414.78</u>	k
Lateral Load =	<u>43.04</u>	<u>921.21</u>	k
Moment (Weak Axis) =	<u>0.07</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 25517.5$  in-lbs  
Resisting Force Required = 850.58 lbs  
S.F. = 1.67  
Weight Required = 1417.64 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 221.34 lbs  
Friction = 0.4  
Weight Required = 553.36 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 221.34 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$   
22 in 23 in 24 in 25 in  
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	651 lbs	651 lbs	651 lbs	651 lbs	493 lbs	493 lbs	493 lbs	493 lbs	805 lbs	805 lbs	805 lbs	805 lbs	-70 lbs	-70 lbs	-70 lbs	-70 lbs
$F_B$	472 lbs	472 lbs	472 lbs	472 lbs	533 lbs	533 lbs	533 lbs	533 lbs	715 lbs	715 lbs	715 lbs	715 lbs	-824 lbs	-824 lbs	-824 lbs	-824 lbs
$F_V$	72 lbs	72 lbs	72 lbs	72 lbs	402 lbs	402 lbs	402 lbs	402 lbs	350 lbs	350 lbs	350 lbs	350 lbs	-443 lbs	-443 lbs	-443 lbs	-443 lbs
$P_{total}$	3117 lbs	3207 lbs	3298 lbs	3389 lbs	3020 lbs	3111 lbs	3201 lbs	3292 lbs	3513 lbs	3604 lbs	3695 lbs	3785 lbs	302 lbs	356 lbs	411 lbs	465 lbs
$M$	457 lbs-ft	457 lbs-ft	457 lbs-ft	457 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	722 lbs-ft	722 lbs-ft	722 lbs-ft	722 lbs-ft	690 lbs-ft	690 lbs-ft	690 lbs-ft	690 lbs-ft
$e$	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.28 ft	1.93 ft	1.68 ft	1.48 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	280.1 psf	277.4 psf	274.9 psf	272.6 psf	257.3 psf	255.6 psf	254.0 psf	252.5 psf	288.8 psf	285.7 psf	282.8 psf	280.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	399.9 psf	391.9 psf	384.7 psf	378.0 psf	401.6 psf	393.6 psf	386.3 psf	379.5 psf	477.8 psf	466.5 psf	456.1 psf	446.6 psf	504.8 psf	219.2 psf	166.7 psf	146.3 psf

Maximum Bearing Pressure = 505 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

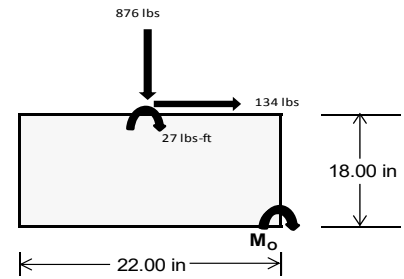
### Overturning Check

$M_o = 575.6 \text{ ft-lbs}$   
 Resisting Force Required = 627.98 lbs  
 S.F. = 1.67  
 Weight Required = 1046.63 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	141 lbs	169 lbs	85 lbs	353 lbs	876 lbs	310 lbs	80 lbs	8 lbs	28 lbs
$F_v$	23 lbs	177 lbs	23 lbs	15 lbs	134 lbs	18 lbs	23 lbs	177 lbs	23 lbs
$P_{total}$	2609 lbs	2637 lbs	2553 lbs	2703 lbs	3226 lbs	2660 lbs	802 lbs	729 lbs	749 lbs
$M$	65 lbs-ft	301 lbs-ft	70 lbs-ft	43 lbs-ft	228 lbs-ft	55 lbs-ft	67 lbs-ft	301 lbs-ft	70 lbs-ft
$e$	0.02 ft	0.11 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.08 ft	0.41 ft	0.09 ft
$L/6$	0.31 ft	1.60 ft	1.78 ft	1.80 ft	1.69 ft	1.79 ft	1.67 ft	1.01 ft	1.65 ft
$f_{min}$	261.4 sqft	180.2 sqft	253.4 sqft	279.7 sqft	270.6 sqft	270.4 sqft	63.7 sqft	-27.8 sqft	56.7 sqft
$f_{max}$	307.8 psf	395.2 psf	303.6 psf	310.0 psf	433.3 psf	309.9 psf	111.2 psf	187.0 psf	106.7 psf



Maximum Bearing Pressure = 433 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.415 k
Allowable Uplift =	1.214 k
Utilization =	<u>34%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.308 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

#### Diagonal Strut

Maximum Axial Load =	0.476 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



#### Rear Strut

Maximum Axial Load =	1.184 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

#### Bracing

Maximum Axial Load =	0.245 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	30.83 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.617 in
Max Drift, $\Delta_{MAX}$ =	0.112 in
	<u>0.112 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 90.00 \text{ in}$$

$$J = \frac{0.255}{234.355}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 90.00 \text{ in}$$

$$J = \frac{0.255}{243.363}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.207 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &21.0912 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &24.5845 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



# A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

## 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

## 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

## 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

## 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

## 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

## 3.4.16.1

N/A for Weak Direction

## 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

## 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

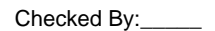
$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 25° 90mph 30psf 7.5ft 7-05.r3d Page 20





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	333.511	1	.028	2	.659	1	0	12	.001	1	0	15
30			min	-359.886	3	-.029	3	-.337	5	-.001	1	0	3	0	6
31		16	max	333.628	1	-.007	10	.659	1	0	12	.001	1	0	15
32			min	-359.799	3	-.056	3	-.442	5	-.001	1	0	3	0	6
33		17	max	333.744	1	-.022	15	.659	1	0	12	.002	1	0	15
34			min	-359.712	3	-.092	4	-.548	5	-.001	1	0	3	0	6
35		18	max	333.861	1	-.033	15	.659	1	0	12	.002	1	0	15
36			min	-359.625	3	-.138	4	-.653	5	-.001	1	0	3	0	6
37		19	max	333.977	1	-.044	15	.659	1	0	12	.002	1	0	15
38			min	-359.537	3	-.183	4	-.759	5	-.001	1	0	3	0	6
39	M3	1	max	105.51	2	1.775	6	-.035	12	0	5	.002	1	0	6
40			min	-128.812	3	.417	15	-1.449	4	0	1	0	12	0	15
41		2	max	105.441	2	1.597	6	-.035	12	0	5	.002	1	0	2
42			min	-128.863	3	.375	15	-1.315	4	0	1	0	12	0	15
43		3	max	105.372	2	1.42	6	-.035	12	0	5	.002	1	0	2
44			min	-128.915	3	.333	15	-1.181	4	0	1	0	15	0	3
45		4	max	105.304	2	1.243	6	-.035	12	0	5	.002	1	0	15
46			min	-128.966	3	.292	15	-1.048	4	0	1	0	5	0	4
47		5	max	105.235	2	1.066	6	-.035	12	0	5	.002	1	0	15
48			min	-129.018	3	.25	15	-.914	4	0	1	0	5	0	4
49		6	max	105.167	2	.889	6	-.035	12	0	5	.001	1	0	15
50			min	-129.069	3	.208	15	-.781	4	0	1	0	5	0	4
51		7	max	105.098	2	.711	6	-.035	12	0	5	.001	1	0	15
52			min	-129.121	3	.167	15	-.671	1	0	1	0	5	0	4
53		8	max	105.029	2	.534	6	-.035	12	0	5	.001	1	0	15
54			min	-129.172	3	.125	15	-.671	1	0	1	0	5	-.001	4
55		9	max	104.961	2	.357	6	-.035	12	0	5	0	1	0	15
56			min	-129.223	3	.083	15	-.671	1	0	1	0	5	-.001	4
57		10	max	104.892	2	.18	6	-.035	12	0	5	0	1	0	15
58			min	-129.275	3	.042	15	-.671	1	0	1	0	5	-.001	4
59		11	max	104.824	2	.024	2	.026	5	0	5	0	1	0	15
60			min	-129.326	3	-.021	3	-.671	1	0	1	0	5	-.001	4
61		12	max	104.755	2	-.042	15	.16	5	0	5	0	1	0	15
62			min	-129.378	3	-.175	4	-.671	1	0	1	0	5	-.001	4
63		13	max	104.686	2	-.083	15	.293	5	0	5	0	1	0	15
64			min	-129.429	3	-.352	4	-.671	1	0	1	0	5	-.001	4
65		14	max	104.618	2	-.125	15	.427	5	0	5	0	1	0	15
66			min	-129.481	3	-.529	4	-.671	1	0	1	0	5	-.001	4
67		15	max	104.549	2	-.166	15	.56	5	0	5	0	1	0	15
68			min	-129.532	3	-.706	4	-.671	1	0	1	0	5	0	4
69		16	max	104.481	2	-.208	15	.694	5	0	5	0	10	0	15
70			min	-129.584	3	-.884	4	-.671	1	0	1	0	4	0	4
71		17	max	104.412	2	-.25	15	.828	5	0	5	0	12	0	15
72			min	-129.635	3	-1.061	4	-.671	1	0	1	0	4	0	4
73		18	max	104.343	2	-.291	15	.961	5	0	5	0	15	0	15
74			min	-129.687	3	-1.238	4	-.671	1	0	1	0	1	0	4
75		19	max	104.275	2	-.333	15	1.095	5	0	5	0	5	0	1
76			min	-129.738	3	-1.415	4	-.671	1	0	1	0	1	0	1
77	M4	1	max	478.48	1	0	1	-.17	12	0	1	0	5	0	1
78			min	-27.433	3	0	1	-32.49	4	0	1	0	1	0	1
79		2	max	478.545	1	0	1	-.17	12	0	1	0	12	0	1
80			min	-27.385	3	0	1	-32.546	4	0	1	-.003	4	0	1
81		3	max	478.61	1	0	1	-.17	12	0	1	0	12	0	1
82			min	-27.336	3	0	1	-32.602	4	0	1	-.006	4	0	1
83		4	max	478.675	1	0	1	-.17	12	0	1	0	12	0	1
84			min	-27.287	3	0	1	-32.658	4	0	1	-.009	4	0	1
85		5	max	478.739	1	0	1	-.17	12	0	1	0	12	0	1



Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-27.239	3	0	1	-32.714	4	0	1	-.012	4	0	1
87		6	max	478.804	1	0	1	-.17	12	0	1	0	12	0	1
88			min	-27.19	3	0	1	-32.77	4	0	1	-.015	4	0	1
89		7	max	478.869	1	0	1	-.17	12	0	1	0	12	0	1
90			min	-27.142	3	0	1	-32.827	4	0	1	-.017	4	0	1
91		8	max	478.933	1	0	1	-.17	12	0	1	0	12	0	1
92			min	-27.093	3	0	1	-32.883	4	0	1	-.02	4	0	1
93		9	max	478.998	1	0	1	-.17	12	0	1	0	12	0	1
94			min	-27.045	3	0	1	-32.939	4	0	1	-.023	4	0	1
95		10	max	479.063	1	0	1	-.17	12	0	1	0	12	0	1
96			min	-26.996	3	0	1	-32.995	4	0	1	-.026	4	0	1
97		11	max	479.127	1	0	1	-.17	12	0	1	0	12	0	1
98			min	-26.948	3	0	1	-33.051	4	0	1	-.029	4	0	1
99		12	max	479.192	1	0	1	-.17	12	0	1	0	12	0	1
100			min	-26.899	3	0	1	-33.107	4	0	1	-.032	4	0	1
101		13	max	479.257	1	0	1	-.17	12	0	1	0	12	0	1
102			min	-26.851	3	0	1	-33.163	4	0	1	-.035	4	0	1
103		14	max	479.322	1	0	1	-.17	12	0	1	0	12	0	1
104			min	-26.802	3	0	1	-33.219	4	0	1	-.038	4	0	1
105		15	max	479.386	1	0	1	-.17	12	0	1	0	12	0	1
106			min	-26.754	3	0	1	-33.275	4	0	1	-.041	4	0	1
107		16	max	479.451	1	0	1	-.17	12	0	1	0	12	0	1
108			min	-26.705	3	0	1	-33.331	4	0	1	-.044	4	0	1
109		17	max	479.516	1	0	1	-.17	12	0	1	0	12	0	1
110			min	-26.657	3	0	1	-33.387	4	0	1	-.047	4	0	1
111		18	max	479.58	1	0	1	-.17	12	0	1	0	12	0	1
112			min	-26.608	3	0	1	-33.443	4	0	1	-.05	4	0	1
113		19	max	479.645	1	0	1	-.17	12	0	1	0	12	0	1
114			min	-26.56	3	0	1	-33.499	4	0	1	-.053	4	0	1
115	M6	1	max	1085.263	1	.629	6	1.196	4	0	1	0	3	0	1
116			min	-1184.471	3	.142	15	-.135	3	0	5	0	1	0	1
117		2	max	1085.379	1	.583	6	1.09	4	0	1	0	4	0	15
118			min	-1184.383	3	.132	15	-.135	3	0	5	0	11	0	6
119		3	max	1085.496	1	.538	6	.985	4	0	1	0	4	0	15
120			min	-1184.296	3	.121	15	-.135	3	0	5	0	10	0	6
121		4	max	1085.612	1	.492	6	.879	4	0	1	0	4	0	15
122			min	-1184.209	3	.11	15	-.135	3	0	5	0	10	0	6
123		5	max	1085.729	1	.45	2	.774	4	0	1	0	4	0	15
124			min	-1184.121	3	.099	15	-.135	3	0	5	0	12	0	6
125		6	max	1085.845	1	.415	2	.668	4	0	1	0	4	0	15
126			min	-1184.034	3	.089	15	-.135	3	0	5	0	3	0	6
127		7	max	1085.961	1	.379	2	.563	4	0	1	0	4	0	15
128			min	-1183.947	3	.078	15	-.135	3	0	5	0	3	0	6
129		8	max	1086.078	1	.344	2	.457	4	0	1	.001	4	0	15
130			min	-1183.86	3	.064	12	-.135	3	0	5	0	3	0	6
131		9	max	1086.194	1	.308	2	.352	4	0	1	.001	4	0	15
132			min	-1183.772	3	.047	12	-.135	3	0	5	0	3	0	2
133		10	max	1086.311	1	.272	2	.252	14	0	1	.001	4	0	15
134			min	-1183.685	3	.029	12	-.135	3	0	5	0	3	0	2
135		11	max	1086.427	1	.237	2	.216	1	0	1	.001	4	0	15
136			min	-1183.598	3	.011	3	-.135	3	0	5	0	3	0	2
137		12	max	1086.543	1	.201	2	.216	1	0	1	.001	4	0	15
138			min	-1183.51	3	-.016	3	-.135	3	0	5	0	3	0	2
139		13	max	1086.66	1	.166	2	.216	1	0	1	.001	4	0	15
140			min	-1183.423	3	-.043	3	-.144	5	0	5	0	3	0	2
141		14	max	1086.776	1	.13	2	.216	1	0	1	.001	4	0	15
142			min	-1183.336	3	-.07	3	-.249	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1086.893	1	.095	2	.216	1	0	1	.001	4	0	12
144		min	-1183.248	3	-.096	3	-.354	5	0	5	0	3	0	2
145	16	max	1087.009	1	.059	2	.216	1	0	1	.001	4	0	12
146		min	-1183.161	3	-.123	3	-.46	5	0	5	0	3	0	2
147	17	max	1087.125	1	.023	2	.216	1	0	1	.001	4	0	12
148		min	-1183.074	3	-.15	3	-.565	5	0	5	0	3	0	2
149	18	max	1087.242	1	-.012	2	.216	1	0	1	0	4	0	12
150		min	-1182.987	3	-.176	3	-.671	5	0	5	0	3	0	2
151	19	max	1087.358	1	-.048	2	.216	1	0	1	0	4	0	12
152		min	-1182.899	3	-.203	3	-.776	5	0	5	0	3	0	2
153	M7	1	max	475.718	2	1.788	.014	1	0	2	0	4	0	2
154		min	-398.415	3	.425	15	-1.364	5	0	3	0	3	0	12
155	2	max	475.649	2	1.611	4	.014	1	0	2	0	4	0	2
156		min	-398.466	3	.383	15	-1.23	5	0	3	0	3	0	3
157	3	max	475.581	2	1.434	4	.014	1	0	2	0	4	0	2
158		min	-398.518	3	.341	15	-1.096	5	0	3	0	3	0	3
159	4	max	475.512	2	1.257	4	.014	1	0	2	0	2	0	2
160		min	-398.569	3	.3	15	-.963	5	0	3	0	3	0	3
161	5	max	475.443	2	1.079	4	.014	1	0	2	0	2	0	15
162		min	-398.621	3	.258	15	-.829	5	0	3	0	5	0	3
163	6	max	475.375	2	.902	4	.014	1	0	2	0	2	0	15
164		min	-398.672	3	.216	15	-.695	5	0	3	0	5	0	6
165	7	max	475.306	2	.725	4	.014	1	0	2	0	2	0	15
166		min	-398.724	3	.175	15	-.562	5	0	3	0	5	0	6
167	8	max	475.238	2	.548	4	.014	1	0	2	0	2	0	15
168		min	-398.775	3	.133	15	-.428	5	0	3	0	5	-.001	6
169	9	max	475.169	2	.371	4	.014	1	0	2	0	2	0	15
170		min	-398.827	3	.087	12	-.295	5	0	3	0	5	-.001	6
171	10	max	475.1	2	.224	2	.014	1	0	2	0	2	0	15
172		min	-398.878	3	.018	12	-.161	5	0	3	0	5	-.001	6
173	11	max	475.032	2	.086	2	.014	1	0	2	0	2	0	15
174		min	-398.93	3	-.082	3	-.027	5	0	3	0	5	-.001	6
175	12	max	474.963	2	-.034	15	.11	4	0	2	0	2	0	15
176		min	-398.981	3	-.186	3	-.004	10	0	3	0	5	-.001	6
177	13	max	474.895	2	-.075	15	.244	4	0	2	0	2	0	15
178		min	-399.032	3	-.339	6	-.004	10	0	3	0	5	-.001	6
179	14	max	474.826	2	-.117	15	.377	4	0	2	0	2	0	15
180		min	-399.084	3	-.516	6	-.004	10	0	3	0	5	-.001	6
181	15	max	474.757	2	-.158	15	.511	4	0	2	0	2	0	15
182		min	-399.135	3	-.693	6	-.004	10	0	3	0	5	0	6
183	16	max	474.689	2	-.2	15	.644	4	0	2	0	2	0	15
184		min	-399.187	3	-.87	6	-.004	10	0	3	0	5	0	6
185	17	max	474.62	2	-.242	15	.778	4	0	2	0	2	0	15
186		min	-399.238	3	-1.047	6	-.004	10	0	3	0	5	0	6
187	18	max	474.552	2	-.283	15	.912	4	0	2	0	2	0	15
188		min	-399.29	3	-1.225	6	-.004	10	0	3	0	5	0	6
189	19	max	474.483	2	-.325	15	1.045	4	0	2	0	14	0	1
190		min	-399.341	3	-1.402	6	-.004	10	0	3	0	3	0	1
191	M8	1	max	1307.136	1	0	.766	1	0	1	0	4	0	1
192		min	-117.761	3	0	1	-32.571	4	0	1	0	1	0	1
193	2	max	1307.201	1	0	1	.766	1	0	1	0	1	0	1
194		min	-117.712	3	0	1	-32.627	4	0	1	-.003	4	0	1
195	3	max	1307.266	1	0	1	.766	1	0	1	0	1	0	1
196		min	-117.664	3	0	1	-32.683	4	0	1	-.006	4	0	1
197	4	max	1307.33	1	0	1	.766	1	0	1	0	1	0	1
198		min	-117.615	3	0	1	-32.739	4	0	1	-.009	4	0	1
199	5	max	1307.395	1	0	1	.766	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-117.566	3	0	1	-32.795	4	0	1	-.012	4	0	1
201		6	max	1307.46	1	0	1	.766	1	0	1	0	1	0	1
202			min	-117.518	3	0	1	-32.851	4	0	1	-.015	4	0	1
203		7	max	1307.525	1	0	1	.766	1	0	1	0	1	0	1
204			min	-117.469	3	0	1	-32.907	4	0	1	-.018	4	0	1
205		8	max	1307.589	1	0	1	.766	1	0	1	0	1	0	1
206			min	-117.421	3	0	1	-32.963	4	0	1	-.02	4	0	1
207		9	max	1307.654	1	0	1	.766	1	0	1	0	1	0	1
208			min	-117.372	3	0	1	-33.02	4	0	1	-.023	4	0	1
209		10	max	1307.719	1	0	1	.766	1	0	1	0	1	0	1
210			min	-117.324	3	0	1	-33.076	4	0	1	-.026	4	0	1
211		11	max	1307.783	1	0	1	.766	1	0	1	0	1	0	1
212			min	-117.275	3	0	1	-33.132	4	0	1	-.029	4	0	1
213		12	max	1307.848	1	0	1	.766	1	0	1	0	1	0	1
214			min	-117.227	3	0	1	-33.188	4	0	1	-.032	4	0	1
215		13	max	1307.913	1	0	1	.766	1	0	1	0	1	0	1
216			min	-117.178	3	0	1	-33.244	4	0	1	-.035	4	0	1
217		14	max	1307.977	1	0	1	.766	1	0	1	0	1	0	1
218			min	-117.13	3	0	1	-33.3	4	0	1	-.038	4	0	1
219		15	max	1308.042	1	0	1	.766	1	0	1	0	1	0	1
220			min	-117.081	3	0	1	-33.356	4	0	1	-.041	4	0	1
221		16	max	1308.107	1	0	1	.766	1	0	1	.001	1	0	1
222			min	-117.033	3	0	1	-33.412	4	0	1	-.044	4	0	1
223		17	max	1308.172	1	0	1	.766	1	0	1	.001	1	0	1
224			min	-116.984	3	0	1	-33.468	4	0	1	-.047	4	0	1
225		18	max	1308.236	1	0	1	.766	1	0	1	.001	1	0	1
226			min	-116.936	3	0	1	-33.524	4	0	1	-.05	4	0	1
227		19	max	1308.301	1	0	1	.766	1	0	1	.001	1	0	1
228			min	-116.887	3	0	1	-33.58	4	0	1	-.053	4	0	1
229	M10	1	max	344.749	1	.665	4	1.385	5	.001	1	0	1	0	1
230			min	-343.886	3	.168	15	-.188	1	-.002	5	0	3	0	1
231		2	max	344.865	1	.62	4	1.279	5	.001	1	0	1	0	15
232			min	-343.799	3	.157	15	-.188	1	-.002	5	0	3	0	4
233		3	max	344.981	1	.574	4	1.174	5	.001	1	0	4	0	15
234			min	-343.711	3	.146	15	-.188	1	-.002	5	0	3	0	4
235		4	max	345.098	1	.528	4	1.068	5	.001	1	0	4	0	15
236			min	-343.624	3	.135	15	-.188	1	-.002	5	0	3	0	4
237		5	max	345.214	1	.483	4	.963	5	.001	1	0	4	0	15
238			min	-343.537	3	.125	15	-.188	1	-.002	5	0	3	0	4
239		6	max	345.331	1	.437	4	.857	5	.001	1	0	4	0	15
240			min	-343.45	3	.114	15	-.188	1	-.002	5	0	3	0	4
241		7	max	345.447	1	.391	4	.752	5	.001	1	.001	4	0	15
242			min	-343.362	3	.103	15	-.188	1	-.002	5	0	3	0	4
243		8	max	345.563	1	.346	4	.646	5	.001	1	.001	4	0	15
244			min	-343.275	3	.092	15	-.188	1	-.002	5	0	3	0	4
245		9	max	345.68	1	.3	4	.541	5	.001	1	.001	4	0	15
246			min	-343.188	3	.082	15	-.188	1	-.002	5	0	3	0	4
247		10	max	345.796	1	.254	4	.436	5	.001	1	.001	4	0	15
248			min	-343.1	3	.071	15	-.188	1	-.002	5	0	3	0	4
249		11	max	345.913	1	.209	4	.33	5	.001	1	.001	4	0	15
250			min	-343.013	3	.06	15	-.188	1	-.002	5	0	3	0	4
251		12	max	346.029	1	.163	4	.225	5	.001	1	.001	4	0	15
252			min	-342.926	3	.049	15	-.188	1	-.002	5	0	3	0	4
253		13	max	346.145	1	.117	4	.119	5	.001	1	.001	4	0	15
254			min	-342.838	3	.031	1	-.188	1	-.002	5	0	1	0	4
255		14	max	346.262	1	.072	4	.014	5	.001	1	.001	4	0	15
256			min	-342.751	3	-.004	1	-.188	1	-.002	5	0	1	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	346.378	1	.028	2	-.009	12	.001	1	.001	4	0	15
258			min	-342.664	3	-.04	1	-.188	1	-.002	5	0	1	0	4
259		16	max	346.495	1	.009	5	-.009	12	.001	1	.001	4	0	15
260			min	-342.577	3	-.076	1	-.218	4	-.002	5	0	1	0	4
261		17	max	346.611	1	-.004	15	-.009	12	.001	1	.001	4	0	15
262			min	-342.489	3	-.111	1	-.323	4	-.002	5	0	1	0	4
263		18	max	346.727	1	-.015	15	-.009	12	.001	1	.001	4	0	15
264			min	-342.402	3	-.147	1	-.428	4	-.002	5	0	1	0	4
265		19	max	346.844	1	-.026	15	-.009	12	.001	1	.001	5	0	15
266			min	-342.315	3	-.182	1	-.534	4	-.002	5	0	1	0	4
267	M11	1	max	105.247	2	1.768	6	.775	1	.002	4	.001	5	0	6
268			min	-129.436	3	.412	15	-1.163	5	0	10	-.002	1	0	15
269		2	max	105.178	2	1.591	6	.775	1	.002	4	0	5	0	1
270			min	-129.487	3	.371	15	-1.029	5	0	10	-.002	1	0	12
271		3	max	105.109	2	1.414	6	.775	1	.002	4	0	5	0	1
272			min	-129.539	3	.329	15	-.896	5	0	10	-.002	1	0	3
273		4	max	105.041	2	1.237	6	.775	1	.002	4	0	5	0	15
274			min	-129.59	3	.287	15	-.762	5	0	10	-.002	1	0	4
275		5	max	104.972	2	1.059	6	.775	1	.002	4	0	5	0	15
276			min	-129.642	3	.246	15	-.628	5	0	10	-.001	1	0	4
277		6	max	104.904	2	.882	6	.775	1	.002	4	0	5	0	15
278			min	-129.693	3	.204	15	-.495	5	0	10	-.001	1	0	4
279		7	max	104.835	2	.705	6	.775	1	.002	4	0	5	0	15
280			min	-129.745	3	.162	15	-.361	5	0	10	-.001	1	0	4
281		8	max	104.766	2	.528	6	.775	1	.002	4	0	3	0	15
282			min	-129.796	3	.121	15	-.227	5	0	10	0	1	-.001	4
283		9	max	104.698	2	.351	6	.775	1	.002	4	0	3	0	15
284			min	-129.847	3	.079	15	-.094	5	0	10	0	1	-.001	4
285		10	max	104.629	2	.173	6	.775	1	.002	4	0	3	0	15
286			min	-129.899	3	.037	15	.01	12	0	10	0	1	-.001	4
287		11	max	104.561	2	.025	1	.775	1	.002	4	0	3	0	15
288			min	-129.95	3	-.04	3	.01	12	0	10	0	1	-.001	4
289		12	max	104.492	2	-.046	15	.775	1	.002	4	0	3	0	15
290			min	-130.002	3	-.181	4	.01	12	0	10	0	1	-.001	4
291		13	max	104.423	2	-.088	15	.775	1	.002	4	0	5	0	15
292			min	-130.053	3	-.359	4	.01	12	0	10	0	1	-.001	4
293		14	max	104.355	2	-.129	15	.775	1	.002	4	0	4	0	15
294			min	-130.105	3	-.536	4	.01	12	0	10	0	2	-.001	4
295		15	max	104.286	2	-.171	15	.866	4	.002	4	0	4	0	15
296			min	-130.156	3	-.713	4	.01	12	0	10	0	10	0	4
297		16	max	104.218	2	-.213	15	.999	4	.002	4	0	4	0	15
298			min	-130.208	3	-.89	4	.01	12	0	10	0	10	0	4
299		17	max	104.149	2	-.254	15	1.133	4	.002	4	0	4	0	15
300			min	-130.259	3	-1.067	4	.01	12	0	10	0	10	0	4
301		18	max	104.08	2	-.296	15	1.267	4	.002	4	.001	4	0	15
302			min	-130.311	3	-1.245	4	.01	12	0	10	0	10	0	4
303		19	max	104.012	2	-.338	15	1.4	4	.002	4	.002	4	0	1
304			min	-130.362	3	-1.422	4	.01	12	0	10	0	10	0	1
305	M12	1	max	478.177	1	0	1	3.947	1	0	1	0	4	0	1
306			min	-26.95	3	0	1	-29.775	5	0	1	0	3	0	1
307		2	max	478.242	1	0	1	3.947	1	0	1	0	1	0	1
308			min	-26.902	3	0	1	-29.831	5	0	1	-.003	5	0	1
309		3	max	478.307	1	0	1	3.947	1	0	1	0	1	0	1
310			min	-26.853	3	0	1	-29.887	5	0	1	-.005	5	0	1
311		4	max	478.371	1	0	1	3.947	1	0	1	.001	1	0	1
312			min	-26.805	3	0	1	-29.943	5	0	1	-.008	5	0	1
313		5	max	478.436	1	0	1	3.947	1	0	1	.001	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-26.756	3	0	1	-29.999	5	0	1	-.011	5	0	1
315		6	max	478.501	1	0	1	3.947	1	0	1	.002	1	0	1
316			min	-26.708	3	0	1	-30.055	5	0	1	-.013	5	0	1
317		7	max	478.565	1	0	1	3.947	1	0	1	.002	1	0	1
318			min	-26.659	3	0	1	-30.111	5	0	1	-.016	5	0	1
319		8	max	478.63	1	0	1	3.947	1	0	1	.003	1	0	1
320			min	-26.611	3	0	1	-30.167	5	0	1	-.019	5	0	1
321		9	max	478.695	1	0	1	3.947	1	0	1	.003	1	0	1
322			min	-26.562	3	0	1	-30.224	5	0	1	-.021	5	0	1
323		10	max	478.76	1	0	1	3.947	1	0	1	.003	1	0	1
324			min	-26.514	3	0	1	-30.28	5	0	1	-.024	5	0	1
325		11	max	478.824	1	0	1	3.947	1	0	1	.004	1	0	1
326			min	-26.465	3	0	1	-30.336	5	0	1	-.027	5	0	1
327		12	max	478.889	1	0	1	3.947	1	0	1	.004	1	0	1
328			min	-26.417	3	0	1	-30.392	5	0	1	-.03	5	0	1
329		13	max	478.954	1	0	1	3.947	1	0	1	.004	1	0	1
330			min	-26.368	3	0	1	-30.448	5	0	1	-.032	5	0	1
331		14	max	479.018	1	0	1	3.947	1	0	1	.005	1	0	1
332			min	-26.319	3	0	1	-30.504	5	0	1	-.035	5	0	1
333		15	max	479.083	1	0	1	3.947	1	0	1	.005	1	0	1
334			min	-26.271	3	0	1	-30.56	5	0	1	-.038	5	0	1
335		16	max	479.148	1	0	1	3.947	1	0	1	.005	1	0	1
336			min	-26.222	3	0	1	-30.616	5	0	1	-.04	5	0	1
337		17	max	479.213	1	0	1	3.947	1	0	1	.006	1	0	1
338			min	-26.174	3	0	1	-30.672	5	0	1	-.043	5	0	1
339		18	max	479.277	1	0	1	3.947	1	0	1	.006	1	0	1
340			min	-26.125	3	0	1	-30.728	5	0	1	-.046	5	0	1
341		19	max	479.342	1	0	1	3.947	1	0	1	.006	1	0	1
342			min	-26.077	3	0	1	-30.784	5	0	1	-.049	5	0	1
343	M1	1	max	142.908	1	339.859	3	-3.461	12	0	1	.155	1	0	1
344			min	5.732	12	-329.79	1	-78.462	1	0	3	.007	12	0	3
345		2	max	143.026	1	339.669	3	-3.461	12	0	1	.138	1	.072	1
346			min	5.791	12	-330.043	1	-78.462	1	0	3	.007	12	-.074	3
347		3	max	97.388	1	7.174	9	-3.492	12	0	12	.12	1	.142	1
348			min	-2.767	10	-18.093	3	-78.276	1	0	1	.006	12	-.146	3
349		4	max	97.506	1	6.964	9	-3.492	12	0	12	.103	1	.142	1
350			min	-2.668	10	-18.283	3	-78.276	1	0	1	.005	12	-.142	3
351		5	max	97.625	1	6.753	9	-3.492	12	0	12	.086	1	.143	1
352			min	-2.57	10	-18.473	3	-78.276	1	0	1	.004	12	-.138	3
353		6	max	97.743	1	6.542	9	-3.492	12	0	12	.069	1	.143	1
354			min	-2.472	10	-18.662	3	-78.276	1	0	1	.004	12	-.134	3
355		7	max	97.861	1	6.331	9	-3.492	12	0	12	.052	1	.143	1
356			min	-2.373	10	-18.89	2	-78.276	1	0	1	.003	12	-.13	3
357		8	max	97.979	1	6.12	9	-3.492	12	0	12	.035	1	.144	1
358			min	-2.275	10	-19.144	2	-78.276	1	0	1	.002	12	-.126	3
359		9	max	98.097	1	5.909	9	-3.492	12	0	12	.018	1	.144	1
360			min	-2.177	10	-19.397	2	-78.276	1	0	1	.001	12	-.122	3
361		10	max	98.215	1	5.698	9	-3.492	12	0	12	.003	4	.146	2
362			min	-2.078	10	-19.65	2	-78.276	1	0	1	0	10	-.118	3
363		11	max	98.333	1	5.487	9	-3.492	12	0	12	0	12	.15	2
364			min	-1.98	10	-19.903	2	-78.276	1	0	1	-.016	1	-.113	3
365		12	max	98.451	1	5.276	9	-3.492	12	0	12	-.001	12	.154	2
366			min	-1.882	10	-20.156	2	-78.276	1	0	1	-.033	1	-.109	3
367		13	max	98.569	1	5.065	9	-3.492	12	0	12	-.002	12	.159	2
368			min	-1.783	10	-20.409	2	-78.276	1	0	1	-.05	1	-.105	3
369		14	max	98.687	1	4.855	9	-3.492	12	0	12	-.003	12	.163	2
370			min	-1.685	10	-20.662	2	-78.276	1	0	1	-.067	1	-.1	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	98.805	1	4.644	9	-3.492	12	0	12	-.003	12	.168	2
372			min	-1.587	10	-20.915	2	-78.276	1	0	1	-.084	1	-.096	3
373		16	max	88.132	2	60.01	2	-3.528	12	0	1	-.004	12	.172	2
374			min	-19.725	3	-122.464	3	-78.849	1	0	5	-.102	1	-.091	3
375		17	max	88.25	2	59.757	2	-3.528	12	0	1	-.005	12	.162	1
376			min	-19.636	3	-122.654	3	-78.849	1	0	5	-.119	1	-.064	3
377		18	max	-5.296	12	377.089	1	-3.699	12	0	3	-.006	12	.082	1
378			min	-142.525	1	-147.738	3	-80.836	1	0	1	-.136	1	-.032	3
379		19	max	-5.237	12	376.836	1	-3.699	12	0	3	-.006	12	0	1
380			min	-142.407	1	-147.928	3	-80.836	1	0	1	-.154	1	0	3
381	M5	1	max	311.542	1	1125.53	3	-.067	10	0	1	.043	4	0	3
382			min	8.605	15	-1093.621	1	-34.981	3	0	5	0	10	0	1
383		2	max	311.66	1	1125.341	3	-.067	10	0	1	.037	4	.237	1
384			min	8.641	15	-1093.874	1	-34.981	3	0	5	-.004	3	-.244	3
385		3	max	181.933	3	7.375	9	4.05	3	0	3	.031	4	.47	1
386			min	-23.057	10	-70.898	2	-23.068	4	0	4	-.011	3	-.483	3
387		4	max	182.021	3	7.165	9	4.05	3	0	3	.026	4	.475	1
388			min	-22.958	10	-71.151	2	-22.826	4	0	4	-.01	3	-.469	3
389		5	max	182.11	3	6.954	9	4.05	3	0	3	.021	4	.481	1
390			min	-22.86	10	-71.405	2	-22.584	4	0	4	-.009	3	-.454	3
391		6	max	182.198	3	6.743	9	4.05	3	0	3	.016	4	.487	1
392			min	-22.761	10	-71.658	2	-22.342	4	0	4	-.008	3	-.44	3
393		7	max	182.287	3	6.532	9	4.05	3	0	3	.011	4	.493	1
394			min	-22.663	10	-71.911	2	-22.1	4	0	4	-.008	3	-.426	3
395		8	max	182.375	3	6.321	9	4.05	3	0	3	.007	4	.499	1
396			min	-22.565	10	-72.164	2	-21.858	4	0	4	-.007	3	-.412	3
397		9	max	182.464	3	6.11	9	4.05	3	0	3	.002	5	.506	1
398			min	-22.466	10	-72.417	2	-21.616	4	0	4	-.006	3	-.397	3
399		10	max	182.552	3	5.899	9	4.05	3	0	3	0	10	.512	1
400			min	-22.368	10	-72.67	2	-21.374	4	0	4	-.005	3	-.383	3
401		11	max	182.641	3	5.688	9	4.05	3	0	3	0	10	.518	1
402			min	-22.27	10	-72.923	2	-21.132	4	0	4	-.007	4	-.369	3
403		12	max	182.729	3	5.477	9	4.05	3	0	3	0	10	.528	2
404			min	-22.171	10	-73.176	2	-20.89	4	0	4	-.012	4	-.354	3
405		13	max	182.818	3	5.266	9	4.05	3	0	3	0	10	.543	2
406			min	-22.073	10	-73.429	2	-20.648	4	0	4	-.016	4	-.34	3
407		14	max	182.906	3	5.056	9	4.05	3	0	3	0	10	.559	2
408			min	-21.975	10	-73.682	2	-20.406	4	0	4	-.021	4	-.325	3
409		15	max	182.995	3	4.845	9	4.05	3	0	3	0	10	.575	2
410			min	-21.876	10	-73.935	2	-20.164	4	0	4	-.025	4	-.31	3
411		16	max	306.77	2	300.095	2	4.021	3	0	1	0	12	.588	2
412			min	-65.395	3	-379.768	3	-18.891	4	0	4	-.03	4	-.293	3
413		17	max	306.888	2	299.842	2	4.021	3	0	1	0	3	.534	1
414			min	-65.307	3	-379.958	3	-18.649	4	0	4	-.034	4	-.211	3
415		18	max	-10.429	12	1243.951	1	3.682	3	0	4	.002	3	.269	1
416			min	-312.322	1	-487.323	3	-49.775	5	0	1	-.044	4	-.105	3
417		19	max	-10.37	12	1243.698	1	3.682	3	0	4	.002	3	0	3
418			min	-312.204	1	-487.513	3	-49.533	5	0	1	-.055	4	0	1
419	M9	1	max	142.26	1	339.838	3	208.818	4	0	3	-.002	15	0	1
420			min	3.241	15	-329.773	1	7.203	10	0	1	-.154	1	0	3
421		2	max	142.378	1	339.648	3	209.06	4	0	3	.039	5	.072	1
422			min	3.277	15	-330.026	1	7.203	10	0	1	-.132	1	-.074	3
423		3	max	97.336	1	7.151	9	74.174	1	0	1	.078	5	.142	1
424			min	-2.265	10	-18.03	3	-30.053	5	0	5	-.109	1	-.146	3
425		4	max	97.454	1	6.94	9	74.174	1	0	1	.072	5	.142	1
426			min	-2.167	10	-18.22	3	-29.811	5	0	5	-.093	1	-.142	3
427		5	max	97.572	1	6.729	9	74.174	1	0	1	.065	5	.143	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-2.068	10	-18.409	3	-29.569	5	0	5	-.077	1	-.138	3
429	6	max	97.69	1	6.518	9	74.174	1	0	1	.059	5	.143	1
430		min	-1.97	10	-18.648	2	-29.327	5	0	5	-.061	1	-.134	3
431	7	max	97.808	1	6.307	9	74.174	1	0	1	.052	5	.143	1
432		min	-1.872	10	-18.901	2	-29.085	5	0	5	-.045	1	-.13	3
433	8	max	97.926	1	6.096	9	74.174	1	0	1	.046	5	.144	1
434		min	-1.773	10	-19.154	2	-28.843	5	0	5	-.029	1	-.126	3
435	9	max	98.044	1	5.885	9	74.174	1	0	1	.04	5	.144	1
436		min	-1.675	10	-19.407	2	-28.601	5	0	5	-.013	1	-.122	3
437	10	max	98.162	1	5.674	9	74.174	1	0	1	.034	4	.146	2
438		min	-1.577	10	-19.66	2	-28.359	5	0	5	0	2	-.118	3
439	11	max	98.28	1	5.463	9	74.174	1	0	1	.031	4	.15	2
440		min	-1.478	10	-19.914	2	-28.117	5	0	5	.002	10	-.113	3
441	12	max	98.398	1	5.253	9	74.174	1	0	1	.036	1	.154	2
442		min	-1.38	10	-20.167	2	-27.875	5	0	5	.003	10	-.109	3
443	13	max	98.516	1	5.042	9	74.174	1	0	1	.052	1	.159	2
444		min	-1.282	10	-20.42	2	-27.633	5	0	5	.005	12	-.105	3
445	14	max	98.634	1	4.831	9	74.174	1	0	1	.068	1	.163	2
446		min	-1.183	10	-20.673	2	-27.391	5	0	5	.005	12	-.101	3
447	15	max	98.752	1	4.62	9	74.174	1	0	1	.084	1	.168	2
448		min	-1.085	10	-20.926	2	-27.149	5	0	5	.002	15	-.096	3
449	16	max	88.415	2	59.795	2	74.861	1	0	10	.101	1	.172	2
450		min	-19.813	3	-122.9	3	-25.671	5	0	4	0	15	-.091	3
451	17	max	88.533	2	59.542	2	74.861	1	0	10	.118	1	.162	1
452		min	-19.724	3	-123.09	3	-25.429	5	0	4	-.006	5	-.064	3
453	18	max	.936	5	377.089	1	78.85	1	0	1	.135	1	.082	1
454		min	-142.216	1	-147.736	3	-54.872	5	0	3	-.017	5	-.032	3
455	19	max	.991	5	376.836	1	78.85	1	0	1	.152	1	0	1
456		min	-142.098	1	-147.925	3	-54.63	5	0	3	-.029	5	0	3
457	M13	1	max	208.833	4	329.274	1	-3.241	15	0	.154	1	0	1
458		min	7.206	10	-339.831	3	-142.241	1	0	3	.002	15	0	3
459	2	max	200.649	4	232.174	1	-2.037	15	0	1	.049	1	.241	3
460		min	7.206	10	-239.553	3	-109.09	1	0	3	0	5	-.234	1
461	3	max	192.464	4	135.073	1	-.833	15	0	1	.002	3	.399	3
462		min	7.206	10	-139.276	3	-75.94	1	0	3	-.028	1	-.387	1
463	4	max	184.279	4	37.973	1	.464	5	0	1	0	12	.473	3
464		min	7.206	10	-38.998	3	-42.789	1	0	3	-.077	1	-.459	1
465	5	max	176.095	4	61.279	3	2.326	5	0	1	-.001	15	.464	3
466		min	7.206	10	-59.127	1	-9.638	1	0	3	-.099	1	-.45	1
467	6	max	167.91	4	161.556	3	23.512	1	0	1	0	5	.371	3
468		min	7.206	10	-156.228	1	.25	12	0	3	-.093	1	-.36	1
469	7	max	159.725	4	261.834	3	56.663	1	0	1	.005	5	.195	3
470		min	7.206	10	-253.328	1	1.418	12	0	3	-.06	1	-.19	1
471	8	max	151.54	4	362.111	3	89.813	1	0	1	.011	4	.062	1
472		min	7.206	10	-350.429	1	2.586	12	0	3	0	3	-.065	3
473	9	max	143.356	4	462.388	3	122.964	1	0	1	.09	1	.394	1
474		min	7.206	10	-447.529	1	3.754	12	0	3	.003	12	-.409	3
475	10	max	135.171	4	562.666	3	156.115	1	0	2	.206	1	.808	1
476		min	7.206	10	-544.629	1	4.922	12	0	1	.006	12	-.836	3
477	11	max	100.37	4	447.529	1	.193	15	0	3	.086	1	.394	1
478		min	3.462	12	-462.388	3	-122.312	1	0	1	-.015	5	-.409	3
479	12	max	92.185	4	350.428	1	1.887	5	0	3	.001	2	.062	1
480		min	3.462	12	-362.111	3	-89.162	1	0	1	-.015	4	-.065	3
481	13	max	84	4	253.328	1	3.75	5	0	3	-.004	12	.195	3
482		min	3.462	12	-261.834	3	-56.011	1	0	1	-.062	1	-.19	1
483	14	max	78.733	1	156.228	1	5.612	5	0	3	-.004	12	.371	3
484		min	3.462	12	-161.556	3	-22.861	1	0	1	-.095	1	-.36	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	78.733	1	59.127	1	10.29	1	0	3	-.002	15	.464	3
486			min	3.462	12	-61.279	3	.584	10	0	1	-.1	1	-.45	1
487		16	max	78.733	1	38.999	3	43.441	1	0	3	.004	5	.473	3
488			min	3.462	12	-37.973	1	2.228	12	0	1	-.078	1	-.459	1
489		17	max	78.733	1	139.276	3	76.591	1	0	3	.013	5	.399	3
490			min	3.462	12	-135.073	1	3.396	12	0	1	-.028	1	-.387	1
491		18	max	78.733	1	239.553	3	109.742	1	0	3	.05	1	.241	3
492			min	3.462	12	-232.174	1	4.564	12	0	1	.003	12	-.234	1
493		19	max	78.733	1	339.831	3	142.892	1	0	3	.155	1	0	1
494			min	3.462	12	-329.274	1	5.732	12	0	1	.007	12	0	3
495	M16	1	max	54.627	5	377.371	1	.991	5	0	3	.152	1	0	1
496			min	-78.554	1	-147.947	3	-142.114	1	0	1	-.029	5	0	3
497		2	max	46.442	5	266.084	1	2.854	5	0	3	.047	1	.105	3
498			min	-78.554	1	-104.417	3	-108.963	1	0	1	-.028	5	-.268	1
499		3	max	38.258	5	154.797	1	4.716	5	0	3	0	12	.174	3
500			min	-78.554	1	-60.886	3	-75.813	1	0	1	-.031	4	-.443	1
501		4	max	30.073	5	43.51	1	6.579	5	0	3	-.003	12	.207	3
502			min	-78.554	1	-17.355	3	-42.662	1	0	1	-.079	1	-.526	1
503		5	max	21.888	5	26.175	3	8.441	5	0	3	-.004	12	.203	3
504			min	-78.554	1	-67.777	1	-9.512	1	0	1	-.101	1	-.516	1
505		6	max	13.703	5	69.706	3	23.639	1	0	3	-.004	15	.163	3
506			min	-78.554	1	-179.064	1	.422	12	0	1	-.095	1	-.413	1
507		7	max	5.519	5	113.237	3	56.79	1	0	3	.004	5	.087	3
508			min	-78.554	1	-290.351	1	1.59	12	0	1	-.061	1	-.218	1
509		8	max	-1.386	12	156.767	3	89.94	1	0	3	.015	4	.071	1
510			min	-78.554	1	-401.638	1	2.758	12	0	1	-.002	3	-.026	3
511		9	max	-1.386	12	200.298	3	123.091	1	0	3	.089	1	.452	1
512			min	-78.554	1	-512.925	1	3.926	12	0	1	.001	12	-.175	3
513		10	max	30.528	5	-14.479	15	156.241	1	0	14	.205	1	.926	1
514			min	-80.576	1	-624.212	1	-7.894	3	0	1	.007	12	-.36	3
515		11	max	22.344	5	512.925	1	.251	15	0	1	.089	1	.452	1
516			min	-80.576	1	-200.298	3	-122.782	1	0	3	-.014	5	-.175	3
517		12	max	14.159	5	401.638	1	1.971	5	0	1	.001	2	.071	1
518			min	-80.576	1	-156.767	3	-89.631	1	0	3	-.013	4	-.026	3
519		13	max	5.974	5	290.351	1	3.833	5	0	1	-.002	12	.087	3
520			min	-80.576	1	-113.237	3	-56.48	1	0	3	-.061	1	-.218	1
521		14	max	-1.366	15	179.064	1	5.696	5	0	1	-.003	12	.163	3
522			min	-80.576	1	-69.706	3	-23.33	1	0	3	-.094	1	-.413	1
523		15	max	-3.698	12	67.777	1	9.821	1	0	1	0	15	.203	3
524			min	-80.576	1	-26.175	3	.564	12	0	3	-.1	1	-.516	1
525		16	max	-3.698	12	17.355	3	42.971	1	0	1	.006	5	.207	3
526			min	-80.576	1	-43.51	1	1.732	12	0	3	-.078	1	-.526	1
527		17	max	-3.698	12	60.886	3	76.122	1	0	1	.015	5	.174	3
528			min	-80.576	1	-154.797	1	2.9	12	0	3	-.028	1	-.443	1
529		18	max	-3.698	12	104.417	3	109.273	1	0	1	.049	1	.105	3
530			min	-80.576	1	-266.084	1	4.068	12	0	3	.003	12	-.268	1
531		19	max	-3.698	12	147.947	3	142.423	1	0	1	.154	1	0	1
532			min	-80.576	1	-377.371	1	5.236	12	0	3	.006	12	0	3
533	M15	1	max	0	2	2.039	1	.034	3	0	1	0	1	0	1
534			min	-41.747	3	0	2	-.036	1	0	3	0	3	0	1
535		2	max	0	2	1.813	1	.034	3	0	1	0	1	0	2
536			min	-41.812	3	0	2	-.036	1	0	3	0	3	0	1
537		3	max	0	2	1.586	1	.034	3	0	1	0	1	0	2
538			min	-41.877	3	0	2	-.036	1	0	3	0	3	-.002	1
539		4	max	0	2	1.359	1	.034	3	0	1	0	1	0	2
540			min	-41.942	3	0	2	-.036	1	0	3	0	3	-.002	1
541		5	max	0	2	1.133	1	.034	3	0	1	0	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-42.007	3	0	2	-.036	1	0	3	0	3	-.003	1
543		6	max	0	2	.906	1	.034	3	0	1	0	1	0	2
544			min	-42.073	3	0	2	-.036	1	0	3	0	3	-.003	1
545		7	max	0	2	.68	1	.034	3	0	1	0	3	0	2
546			min	-42.138	3	0	2	-.036	1	0	3	0	1	-.004	1
547		8	max	0	2	.453	1	.034	3	0	1	0	3	0	2
548			min	-42.203	3	0	2	-.036	1	0	3	0	1	-.004	1
549		9	max	0	2	.227	1	.034	3	0	1	0	3	0	2
550			min	-42.268	3	0	2	-.036	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.034	3	0	1	0	3	0	2
552			min	-42.333	3	0	1	-.036	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.034	3	0	1	0	3	0	2
554			min	-42.399	3	-.227	1	-.036	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.034	3	0	1	0	3	0	2
556			min	-42.464	3	-.453	1	-.036	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.034	3	0	1	0	3	0	2
558			min	-42.529	3	-.68	1	-.036	1	0	3	0	1	-.004	1
559		14	max	0	2	0	2	.034	3	0	1	0	3	0	2
560			min	-42.594	3	-.906	1	-.036	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.034	3	0	1	0	3	0	2
562			min	-42.659	3	-1.133	1	-.036	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.034	3	0	1	0	3	0	2
564			min	-42.725	3	-1.359	1	-.036	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.034	3	0	1	0	3	0	2
566			min	-42.79	3	-1.586	1	-.036	1	0	3	0	1	-.002	1
567		18	max	0	2	0	2	.034	3	0	1	0	3	0	2
568			min	-42.855	3	-1.813	1	-.036	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.034	3	0	1	0	3	0	1
570			min	-42.92	3	-2.039	1	-.036	1	0	3	0	1	0	1
571	M16A	1	max	-.853	10	3.29	4	.253	4	0	3	0	3	0	1
572			min	-243.925	4	1.02	12	-.013	3	0	1	0	4	0	1
573		2	max	-.78	10	2.925	4	.228	4	0	3	0	3	0	12
574			min	-244.008	4	.906	12	-.013	3	0	1	0	4	-.001	4
575		3	max	-.708	10	2.559	4	.204	4	0	3	0	3	0	12
576			min	-244.092	4	.793	12	-.013	3	0	1	0	4	-.003	4
577		4	max	-.636	10	2.193	4	.179	4	0	3	0	3	-.001	12
578			min	-244.175	4	.68	12	-.013	3	0	1	0	4	-.004	4
579		5	max	-.563	10	1.828	4	.154	4	0	3	0	3	-.001	12
580			min	-244.259	4	.566	12	-.013	3	0	1	0	1	-.005	4
581		6	max	-.491	10	1.462	4	.13	4	0	3	0	3	-.002	12
582			min	-244.342	4	.453	12	-.013	3	0	1	0	1	-.005	4
583		7	max	-.418	10	1.097	4	.105	4	0	3	0	5	-.002	12
584			min	-244.426	4	.34	12	-.013	3	0	1	0	1	-.006	4
585		8	max	-.346	10	.731	4	.08	4	0	3	0	5	-.002	12
586			min	-244.509	4	.227	12	-.013	3	0	1	0	1	-.006	4
587		9	max	-.273	10	.366	4	.056	4	0	3	0	5	-.002	12
588			min	-244.592	4	.113	12	-.013	3	0	1	0	1	-.007	4
589		10	max	-.201	10	0	1	.031	4	0	3	0	5	-.002	12
590			min	-244.676	4	0	1	-.013	3	0	1	0	1	-.007	4
591		11	max	-.129	10	-.113	12	.022	1	0	3	0	5	-.002	12
592			min	-244.759	4	-.366	4	-.013	3	0	1	0	1	-.007	4
593		12	max	-.056	10	-.227	12	.022	1	0	3	0	5	-.002	12
594			min	-244.843	4	-.731	4	-.022	5	0	1	0	1	-.006	4
595		13	max	.016	10	-.34	12	.022	1	0	3	0	5	-.002	12
596			min	-244.926	4	-1.097	4	-.047	5	0	1	0	3	-.006	4
597		14	max	.089	10	-.453	12	.022	1	0	3	0	4	-.002	12
598			min	-245.01	4	-1.462	4	-.071	5	0	1	0	3	-.005	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.161	10	-566	12	.022	1	0	3	0	4	-.001	12
600		min	-245.093	4	-1.828	4	-.096	5	0	1	0	3	-.005	4
601	16	max	.234	10	-.68	12	.022	1	0	3	0	4	-.001	12
602		min	-245.177	4	-2.193	4	-.121	5	0	1	0	3	-.004	4
603	17	max	.306	10	-.793	12	.022	1	0	3	0	1	0	12
604		min	-245.26	4	-2.559	4	-.145	5	0	1	0	3	-.003	4
605	18	max	.378	10	-.906	12	.022	1	0	3	0	1	0	12
606		min	-245.344	4	-2.925	4	-.17	5	0	1	0	5	-.001	4
607	19	max	.451	10	-1.02	12	.022	1	0	3	0	1	0	1
608		min	-245.427	4	-3.29	4	-.195	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.008	2	.015	1	1.906e-3	5	NC	3	NC	3	
2			min	-.003	3	-.008	3	-.018	5	-1.224e-3	1	4569.43	2	2442.744	1	
3			2	max	.003	1	.007	2	.014	1	1.929e-3	5	NC	3	NC	3
4				min	-.003	3	-.007	3	-.017	5	-1.172e-3	1	4976.945	2	2637.177	1
5			3	max	.003	1	.007	2	.013	1	1.953e-3	5	NC	3	NC	3
6				min	-.003	3	-.007	3	-.016	5	-1.121e-3	1	5459.85	2	2866.512	1
7			4	max	.002	1	.006	2	.012	1	1.976e-3	5	NC	1	NC	3
8				min	-.003	3	-.007	3	-.016	5	-1.069e-3	1	6036.081	2	3139.155	1
9			5	max	.002	1	.005	2	.01	1	1.999e-3	5	NC	1	NC	3
10				min	-.003	3	-.006	3	-.015	5	-1.017e-3	1	6729.476	2	3466.33	1
11			6	max	.002	1	.005	2	.009	1	2.022e-3	5	NC	1	NC	3
12				min	-.002	3	-.006	3	-.014	5	-9.656e-4	1	7572.227	2	3863.291	1
13			7	max	.002	1	.004	2	.008	1	2.046e-3	5	NC	1	NC	2
14				min	-.002	3	-.006	3	-.014	5	-9.139e-4	1	8608.649	2	4351.204	1
15			8	max	.002	1	.004	2	.007	1	2.069e-3	5	NC	1	NC	2
16				min	-.002	3	-.005	3	-.013	5	-8.622e-4	1	9901.104	2	4960.144	1
17			9	max	.002	1	.003	2	.006	1	2.092e-3	5	NC	1	NC	2
18				min	-.002	3	-.005	3	-.012	5	-8.105e-4	1	NC	1	5734.056	1
19			10	max	.001	1	.003	2	.005	1	2.116e-3	5	NC	1	NC	2
20				min	-.002	3	-.005	3	-.011	5	-7.588e-4	1	NC	1	6739.302	1
21		11	max	.001	1	.002	2	.004	1	2.139e-3	5	NC	1	NC	2	
22			min	-.001	3	-.004	3	-.01	5	-7.071e-4	1	NC	1	8080.139	1	
23		12	max	.001	1	.002	2	.004	1	2.162e-3	5	NC	1	NC	2	
24			min	-.001	3	-.004	3	-.009	5	-6.553e-4	1	NC	1	9928.461	1	
25		13	max	0	1	.001	2	.003	1	2.186e-3	5	NC	1	NC	1	
26			min	-.001	3	-.003	3	-.008	5	-6.036e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.209e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.007	5	-5.519e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.232e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.005	5	-5.002e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	2.256e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.004	5	-4.485e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.279e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.003	5	-3.968e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.302e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-3.451e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.325e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.934e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.366e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.082e-3	5	NC	1	NC	1	
41			2	max	0	3	0	2	.006	5	1.701e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-1.094e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.011	5	2.036e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.106e-3	5	NC	1	8761.307	14
45		4	max	0	3	0	2	.017	5	2.371e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	-.001	1	-1.118e-3	5	NC	1	5725.833	14
47		5	max	0	3	0	2	.023	5	2.707e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	-1.13e-3	5	NC	1	4220.82	14
49		6	max	0	3	0	2	.028	4	3.042e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.142e-3	5	NC	1	3326.649	14
51		7	max	0	3	0	2	.034	4	3.377e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.154e-3	5	NC	1	2736.966	14
53		8	max	0	3	0	2	.039	4	3.712e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-1.166e-3	5	NC	1	2320.597	14
55		9	max	0	3	.001	2	.045	4	4.048e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	2	-1.178e-3	5	NC	1	2012.042	14
57		10	max	0	3	.002	2	.051	4	4.383e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-1.19e-3	5	NC	1	1774.968	14
59		11	max	0	3	.002	2	.056	4	4.718e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-1.202e-3	5	NC	1	1587.619	14
61		12	max	0	3	.003	2	.062	4	5.054e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	12	-1.214e-3	5	NC	1	1436.179	14
63		13	max	0	3	.003	2	.067	4	5.389e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	12	-1.226e-3	5	NC	1	1311.459	14
65		14	max	.001	3	.004	2	.072	4	5.724e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	12	-1.238e-3	5	NC	1	1207.112	14
67		15	max	.001	3	.005	2	.077	4	6.059e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	12	-1.25e-3	5	9338.298	2	1118.615	14
69		16	max	.001	3	.006	2	.083	4	6.395e-4	1	NC	1	NC	2
70			min	0	2	-.008	3	0	12	-1.262e-3	5	7891.463	2	1042.658	14
71		17	max	.001	3	.007	2	.088	4	6.73e-4	1	NC	3	NC	2
72			min	-.001	2	-.008	3	0	12	-1.274e-3	5	6776.051	2	976.763	14
73		18	max	.001	3	.008	2	.093	4	7.065e-4	1	NC	3	NC	2
74			min	-.001	2	-.008	3	0	12	-1.286e-3	5	5906.032	2	919.039	14
75		19	max	.001	3	.009	2	.098	4	7.401e-4	1	NC	3	NC	2
76			min	-.001	2	-.008	3	0	12	-1.298e-3	5	5221.162	2	868.015	14
77	M4	1	max	.002	1	.009	2	0	12	5.953e-3	5	NC	1	NC	3
78			min	0	3	-.008	3	-.104	4	-9.749e-4	1	NC	1	186.316	4
79		2	max	.002	1	.009	2	0	12	5.953e-3	5	NC	1	NC	3
80			min	0	3	-.007	3	-.095	4	-9.749e-4	1	NC	1	203.112	4
81		3	max	.002	1	.008	2	0	12	5.953e-3	5	NC	1	NC	2
82			min	0	3	-.007	3	-.087	4	-9.749e-4	1	NC	1	223.104	4
83		4	max	.002	1	.008	2	0	12	5.953e-3	5	NC	1	NC	2
84			min	0	3	-.006	3	-.078	4	-9.749e-4	1	NC	1	247.134	4
85		5	max	.002	1	.007	2	0	12	5.953e-3	5	NC	1	NC	2
86			min	0	3	-.006	3	-.07	4	-9.749e-4	1	NC	1	276.348	4
87		6	max	.002	1	.007	2	0	12	5.953e-3	5	NC	1	NC	2
88			min	0	3	-.005	3	-.062	4	-9.749e-4	1	NC	1	312.341	4
89		7	max	.002	1	.006	2	0	12	5.953e-3	5	NC	1	NC	2
90			min	0	3	-.005	3	-.054	4	-9.749e-4	1	NC	1	357.385	4
91		8	max	.001	1	.006	2	0	12	5.953e-3	5	NC	1	NC	2
92			min	0	3	-.005	3	-.047	4	-9.749e-4	1	NC	1	414.805	4
93		9	max	.001	1	.005	2	0	12	5.953e-3	5	NC	1	NC	2
94			min	0	3	-.004	3	-.039	4	-9.749e-4	1	NC	1	489.632	4
95		10	max	.001	1	.005	2	0	12	5.953e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.033	4	-9.749e-4	1	NC	1	589.773	4
97		11	max	.001	1	.004	2	0	12	5.953e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.027	4	-9.749e-4	1	NC	1	728.253	4
99		12	max	0	1	.004	2	0	12	5.953e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.003	3	-.021	4	-9.749e-4	1	NC	1	927.791	4
101		13	max	0	1	.003	2	0	12	5.953e-3	5	NC	1	NC	1
102			min	0	3	-.003	3	-.016	4	-9.749e-4	1	NC	1	1231.001	4
103		14	max	0	1	.003	2	0	12	5.953e-3	5	NC	1	NC	1
104			min	0	3	-.002	3	-.011	4	-9.749e-4	1	NC	1	1725.785	4
105		15	max	0	1	.002	2	0	12	5.953e-3	5	NC	1	NC	1
106			min	0	3	-.002	3	-.007	4	-9.749e-4	1	NC	1	2618.658	4
107		16	max	0	1	.002	2	0	12	5.953e-3	5	NC	1	NC	1
108			min	0	3	-.001	3	-.004	4	-9.749e-4	1	NC	1	4496.704	4
109		17	max	0	1	.001	2	0	12	5.953e-3	5	NC	1	NC	1
110			min	0	3	0	3	-.002	4	-9.749e-4	1	NC	1	9640.044	4
111		18	max	0	1	0	2	0	12	5.953e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-9.749e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	5.953e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.749e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.029	2	.005	1	2.092e-3	4	NC	3	NC	2
116			min	-.011	3	-.024	3	-.018	5	2.138e-6	10	1238.073	2	7826.214	1
117		2	max	.009	1	.027	2	.004	1	2.111e-3	4	NC	3	NC	2
118			min	-.01	3	-.023	3	-.017	5	1.417e-6	10	1322.718	2	8491.276	1
119		3	max	.009	1	.026	2	.004	1	2.13e-3	4	NC	3	NC	2
120			min	-.009	3	-.022	3	-.017	5	6.95e-7	10	1419.449	2	9279.462	1
121		4	max	.008	1	.024	2	.004	1	2.148e-3	4	NC	3	NC	1
122			min	-.009	3	-.02	3	-.016	5	0	10	1530.681	2	NC	1
123		5	max	.008	1	.022	2	.003	1	2.167e-3	4	NC	3	NC	1
124			min	-.008	3	-.019	3	-.015	5	-7.483e-7	10	1659.524	2	NC	1
125		6	max	.007	1	.02	2	.003	1	2.186e-3	4	NC	3	NC	1
126			min	-.008	3	-.018	3	-.015	5	-2.437e-6	2	1810.049	2	NC	1
127		7	max	.006	1	.018	2	.003	1	2.205e-3	4	NC	3	NC	1
128			min	-.007	3	-.017	3	-.014	5	-5.897e-6	2	1987.688	2	NC	1
129		8	max	.006	1	.017	2	.002	1	2.224e-3	4	NC	3	NC	1
130			min	-.006	3	-.015	3	-.013	5	-9.356e-6	2	2199.851	2	NC	1
131		9	max	.005	1	.015	2	.002	1	2.242e-3	4	NC	3	NC	1
132			min	-.006	3	-.014	3	-.012	5	-1.282e-5	2	2456.914	2	NC	1
133		10	max	.005	1	.013	2	.002	1	2.261e-3	4	NC	3	NC	1
134			min	-.005	3	-.013	3	-.011	5	-1.628e-5	2	2773.865	2	NC	1
135		11	max	.004	1	.011	2	.001	1	2.28e-3	4	NC	3	NC	1
136			min	-.005	3	-.011	3	-.01	5	-1.974e-5	2	3173.186	2	NC	1
137		12	max	.004	1	.01	2	.001	1	2.299e-3	4	NC	3	NC	1
138			min	-.004	3	-.01	3	-.009	5	-2.32e-5	2	3690.201	2	NC	1
139		13	max	.003	1	.008	2	0	1	2.318e-3	4	NC	3	NC	1
140			min	-.004	3	-.009	3	-.008	5	-2.666e-5	2	4383.778	2	NC	1
141		14	max	.003	1	.007	2	0	1	2.336e-3	4	NC	3	NC	1
142			min	-.003	3	-.007	3	-.007	5	-3.011e-5	2	5359.864	2	NC	1
143		15	max	.002	1	.005	2	0	1	2.355e-3	4	NC	3	NC	1
144			min	-.002	3	-.006	3	-.006	5	-3.357e-5	2	6830.333	2	NC	1
145		16	max	.002	1	.004	2	0	1	2.374e-3	4	NC	1	NC	1
146			min	-.002	3	-.004	3	-.004	5	-3.703e-5	2	9289.519	2	NC	1
147		17	max	.001	1	.003	2	0	1	2.393e-3	4	NC	1	NC	1
148			min	-.001	3	-.003	3	-.003	5	-4.049e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.411e-3	4	NC	1	NC	1
150			min	0	3	-.001	3	-.001	5	-4.395e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.43e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.741e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.184e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.131e-3	4	NC	1	NC	1
155		2	max	0	3	.002	2	.006	4	1.893e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-1.128e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.012	4	1.805e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.125e-3	4	NC	1	NC	1
159		4	max	0	3	.004	2	.018	4	1.717e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.122e-3	4	NC	1	NC	1
161		5	max	.001	3	.006	2	.023	4	1.629e-5	1	NC	3	NC	1
162			min	-.001	2	-.007	3	0	1	-1.119e-3	4	7915.461	2	NC	1
163		6	max	.001	3	.007	2	.029	4	1.649e-5	3	NC	3	NC	1
164			min	-.001	2	-.009	3	0	1	-1.116e-3	4	6347.85	2	NC	1
165		7	max	.002	3	.009	2	.035	4	3.061e-5	3	NC	3	NC	1
166			min	-.002	2	-.011	3	0	1	-1.113e-3	4	5277.881	2	NC	1
167		8	max	.002	3	.01	2	.041	4	4.474e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-1.109e-3	4	4494.862	2	NC	1
169		9	max	.002	3	.012	2	.046	4	5.886e-5	3	NC	3	NC	1
170			min	-.002	2	-.014	3	0	1	-1.106e-3	4	3893.779	2	NC	1
171		10	max	.002	3	.013	2	.052	4	7.298e-5	3	NC	3	NC	1
172			min	-.003	2	-.015	3	-.001	1	-1.103e-3	4	3416.428	2	NC	1
173		11	max	.003	3	.015	2	.058	4	8.71e-5	3	NC	3	NC	1
174			min	-.003	2	-.016	3	-.001	1	-1.1e-3	4	3027.879	2	NC	1
175		12	max	.003	3	.017	2	.063	4	1.012e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-1.097e-3	4	2705.811	2	NC	1
177		13	max	.003	3	.019	2	.068	4	1.153e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	-.001	1	-1.094e-3	4	2435.202	2	NC	1
179		14	max	.003	3	.021	2	.073	4	1.295e-4	3	NC	3	NC	1
180			min	-.004	2	-.02	3	-.002	1	-1.091e-3	4	2205.495	2	NC	1
181		15	max	.004	3	.023	2	.079	4	1.436e-4	3	NC	3	NC	1
182			min	-.004	2	-.021	3	-.002	1	-1.088e-3	4	2008.998	2	NC	1
183		16	max	.004	3	.025	2	.084	4	1.577e-4	3	NC	3	NC	1
184			min	-.004	2	-.022	3	-.002	1	-1.085e-3	4	1839.926	2	NC	1
185		17	max	.004	3	.027	2	.089	4	1.718e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	-.002	1	-1.082e-3	4	1693.827	2	NC	1
187		18	max	.004	3	.029	2	.094	4	1.86e-4	3	NC	3	NC	1
188			min	-.005	2	-.023	3	-.002	1	-1.079e-3	4	1567.199	2	NC	1
189		19	max	.005	3	.032	2	.099	4	2.001e-4	3	NC	3	NC	1
190			min	-.005	2	-.024	3	-.002	1	-1.076e-3	4	1457.25	2	NC	1
191	M8	1	max	.006	1	.033	2	.002	1	5.76e-3	4	NC	1	NC	2
192			min	0	3	-.024	3	-.104	4	-1.576e-4	3	NC	1	185.899	4
193		2	max	.006	1	.031	2	.002	1	5.76e-3	4	NC	1	NC	2
194			min	0	3	-.023	3	-.095	4	-1.576e-4	3	NC	1	202.657	4
195		3	max	.006	1	.03	2	.002	1	5.76e-3	4	NC	1	NC	2
196			min	0	3	-.022	3	-.087	4	-1.576e-4	3	NC	1	222.603	4
197		4	max	.005	1	.028	2	.002	1	5.76e-3	4	NC	1	NC	1
198			min	0	3	-.02	3	-.078	4	-1.576e-4	3	NC	1	246.577	4
199		5	max	.005	1	.026	2	.002	1	5.76e-3	4	NC	1	NC	1
200			min	0	3	-.019	3	-.07	4	-1.576e-4	3	NC	1	275.724	4
201		6	max	.004	1	.024	2	.001	1	5.76e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.062	4	-1.576e-4	3	NC	1	311.635	4
203		7	max	.004	1	.022	2	.001	1	5.76e-3	4	NC	1	NC	1
204			min	0	3	-.016	3	-.054	4	-1.576e-4	3	NC	1	356.575	4
205		8	max	.004	1	.02	2	.001	1	5.76e-3	4	NC	1	NC	1
206			min	0	3	-.015	3	-.047	4	-1.576e-4	3	NC	1	413.863	4
207		9	max	.003	1	.019	2	0	1	5.76e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.04	4	-1.576e-4	3	NC	1	488.518	4
209		10	max	.003	1	.017	2	0	1	5.76e-3	4	NC	1	NC	1
210			min	0	3	-.012	3	-.033	4	-1.576e-4	3	NC	1	588.43	4
211		11	max	.003	1	.015	2	0	1	5.76e-3	4	NC	1	NC	1
212			min	0	3	-.011	3	-.027	4	-1.576e-4	3	NC	1	726.592	4
213		12	max	.002	1	.013	2	0	1	5.76e-3	4	NC	1	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.019	5	8.427e-5	10	NC	1	1011.485	5
329		max	0	1	.003	2	.002	1	7.247e-3	4	NC	1	NC	1
330		min	0	3	-.003	3	-.014	5	8.427e-5	10	NC	1	1342.007	5
331		max	0	1	.003	2	.001	1	7.247e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.01	5	8.427e-5	10	NC	1	1881.352	5
333		max	0	1	.002	2	0	1	7.247e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.007	5	8.427e-5	10	NC	1	2854.624	5
335		max	0	1	.002	2	0	1	7.247e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.004	5	8.427e-5	10	NC	1	4901.747	5
337		max	0	1	.001	2	0	1	7.247e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.002	5	8.427e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	7.247e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	8.427e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	7.247e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	8.427e-5	10	NC	1	NC	1
343	M1	max	.007	3	.023	3	.009	5	2.52e-2	1	NC	1	NC	1
344		min	-.008	2	-.024	1	-.005	1	-2.588e-2	3	NC	1	NC	1
345		max	.007	3	.013	3	.013	5	1.213e-2	1	NC	4	NC	2
346		min	-.008	2	-.013	1	-.011	1	-1.282e-2	3	4351.718	1	7735.211	1
347		max	.007	3	.004	3	.018	5	5.81e-4	5	NC	4	NC	2
348		min	-.008	2	-.003	1	-.015	1	-7.032e-4	1	2245.258	1	4691.215	1
349		max	.007	3	.005	1	.023	5	5.894e-4	5	NC	5	NC	2
350		min	-.008	2	-.003	3	-.017	1	-5.947e-4	1	1587.083	1	3410.843	5
351		max	.007	3	.012	1	.028	5	5.978e-4	5	NC	5	NC	3
352		min	-.008	2	-.01	3	-.017	1	-4.863e-4	1	1270.841	1	2452.868	5
353		max	.007	3	.018	1	.034	5	6.061e-4	5	NC	5	NC	2
354		min	-.008	2	-.015	3	-.016	1	-3.778e-4	1	1092.098	1	1891.81	5
355		max	.007	3	.023	1	.04	5	6.145e-4	5	NC	5	NC	2
356		min	-.008	2	-.019	3	-.014	1	-2.694e-4	1	983.848	1	1527.271	5
357		max	.007	3	.026	1	.046	5	6.229e-4	5	NC	5	NC	2
358		min	-.008	2	-.021	3	-.012	1	-1.609e-4	1	918.238	1	1273.767	5
359		max	.007	3	.028	1	.052	5	6.313e-4	5	NC	5	NC	1
360		min	-.008	2	-.023	3	-.008	1	-5.248e-5	1	882.259	1	1084.142	4
361		max	.007	3	.029	1	.058	5	6.476e-4	4	NC	5	NC	1
362		min	-.008	2	-.023	3	-.005	1	1.106e-5	10	869.908	1	929.269	4
363		max	.007	3	.028	2	.065	4	6.801e-4	4	NC	5	NC	1
364		min	-.008	2	-.022	3	-.001	1	2.059e-5	10	874.035	2	812.513	4
365		max	.007	3	.026	2	.072	4	7.127e-4	4	NC	5	NC	2
366		min	-.008	2	-.02	3	0	10	2.936e-5	12	900.254	2	722.516	4
367		max	.007	3	.023	2	.079	4	7.453e-4	4	NC	5	NC	2
368		min	-.008	2	-.017	3	0	12	3.232e-5	12	954.927	2	651.973	4
369		max	.007	3	.018	2	.086	4	7.778e-4	4	NC	5	NC	2
370		min	-.008	2	-.014	3	0	12	3.528e-5	12	1049.908	2	596.016	4
371		max	.007	3	.012	2	.092	4	8.104e-4	4	NC	5	NC	2
372		min	-.008	2	-.009	3	0	12	3.824e-5	12	1211.067	2	551.308	4
373		max	.007	3	.004	1	.098	4	1.178e-3	4	NC	4	NC	2
374		min	-.008	2	-.003	3	0	12	4.021e-5	12	1501.291	2	515.505	4
375		max	.007	3	.003	3	.104	4	9.522e-3	4	NC	4	NC	2
376		min	-.008	2	-.005	2	0	12	2.879e-6	2	2118.668	2	486.965	4
377		max	.007	3	.01	3	.108	4	1.431e-2	1	NC	4	NC	2
378		min	-.008	2	-.016	2	0	10	-5.691e-3	3	4100.219	2	464.447	4
379		max	.007	3	.018	3	.112	4	2.888e-2	1	NC	1	NC	1
380		min	-.008	2	-.028	2	-.003	1	-1.152e-2	3	NC	1	447.603	4
381	M5	max	.022	3	.075	3	.009	5	6.729e-6	4	NC	1	NC	1
382		min	-.028	2	-.083	1	-.006	1	5.172e-8	10	NC	1	NC	1
383		max	.022	3	.043	3	.013	5	2.908e-4	5	NC	5	NC	1
384		min	-.028	2	-.047	1	-.005	1	-7.516e-5	1	1263.538	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.022	3	.013	3	.018	5	5.702e-4	5	NC	5	NC	1
386		min	-.028	2	-.012	1	-.005	1	-1.495e-4	1	651.047	1	NC	1
387	4	max	.022	3	.017	1	.023	5	5.919e-4	5	NC	5	NC	1
388		min	-.028	2	-.011	3	-.004	1	-1.409e-4	1	459.018	1	NC	1
389	5	max	.022	3	.042	1	.029	5	6.136e-4	5	NC	15	NC	1
390		min	-.028	2	-.032	3	-.004	1	-1.323e-4	1	366.58	1	NC	1
391	6	max	.022	3	.063	1	.035	5	6.353e-4	5	NC	15	NC	1
392		min	-.028	2	-.048	3	-.003	1	-1.238e-4	1	314.193	1	NC	1
393	7	max	.022	3	.079	1	.042	5	6.57e-4	5	NC	15	NC	1
394		min	-.028	2	-.061	3	-.003	1	-1.152e-4	1	282.323	1	NC	1
395	8	max	.022	3	.091	1	.048	5	6.787e-4	5	NC	15	NC	1
396		min	-.028	2	-.069	3	-.003	1	-1.066e-4	1	262.841	1	NC	1
397	9	max	.022	3	.098	1	.055	5	7.004e-4	5	NC	15	NC	1
398		min	-.028	2	-.073	3	-.002	1	-9.806e-5	1	251.941	1	NC	1
399	10	max	.022	3	.101	1	.062	5	7.221e-4	5	NC	15	NC	1
400		min	-.028	2	-.074	3	-.002	1	-8.95e-5	1	247.853	1	NC	1
401	11	max	.022	3	.099	1	.068	5	7.438e-4	5	NC	15	NC	1
402		min	-.028	2	-.072	3	-.002	1	-8.093e-5	1	250.016	1	NC	1
403	12	max	.022	3	.092	1	.075	5	7.655e-4	5	NC	15	NC	1
404		min	-.028	2	-.066	3	-.002	1	-7.236e-5	1	258.847	1	NC	1
405	13	max	.022	3	.08	1	.081	4	7.872e-4	5	NC	15	NC	1
406		min	-.028	2	-.057	3	-.002	1	-6.379e-5	1	275.939	1	NC	1
407	14	max	.022	3	.063	1	.087	4	8.089e-4	5	NC	15	NC	1
408		min	-.028	2	-.044	3	-.002	1	-5.523e-5	1	304.819	1	NC	1
409	15	max	.022	3	.042	1	.093	4	8.306e-4	5	NC	5	NC	1
410		min	-.028	2	-.029	3	-.002	1	-4.666e-5	1	353.111	1	NC	1
411	16	max	.022	3	.015	1	.099	4	1.185e-3	5	NC	5	NC	1
412		min	-.028	2	-.011	3	-.002	1	-4.555e-5	1	439.2	1	NC	1
413	17	max	.022	3	.01	3	.104	4	9.512e-3	4	NC	5	NC	1
414		min	-.028	2	-.018	2	-.002	1	-2.219e-4	1	620.002	1	NC	1
415	18	max	.022	3	.033	3	.108	4	4.879e-3	4	NC	5	NC	1
416		min	-.028	2	-.055	2	-.003	1	-1.137e-4	1	1200.428	1	NC	1
417	19	max	.022	3	.057	3	.112	4	2.038e-6	5	NC	1	NC	1
418		min	-.028	2	-.095	2	-.003	1	-1.955e-7	3	NC	1	NC	1
419	M9	1	max	.007	.023	3	.007	5	2.589e-2	3	NC	1	NC	1
420		min	-.008	2	-.024	1	-.007	1	-2.52e-2	1	NC	1	NC	1
421	2	max	.007	3	.013	3	.007	5	1.282e-2	3	NC	4	NC	2
422		min	-.008	2	-.014	1	-.001	1	-1.239e-2	1	4352.688	1	8919.145	1
423	3	max	.007	3	.004	3	.007	4	1.822e-4	1	NC	4	NC	2
424		min	-.008	2	-.004	1	0	3	-8.777e-6	3	2245.771	1	5533.703	1
425	4	max	.007	3	.005	1	.01	4	9.044e-5	1	NC	4	NC	2
426		min	-.008	2	-.004	3	0	3	-1.732e-5	3	1587.437	1	4685.998	1
427	5	max	.007	3	.012	1	.013	4	1.675e-5	4	NC	5	NC	2
428		min	-.008	2	-.01	3	-.001	3	-2.586e-5	3	1271.103	1	4640.487	1
429	6	max	.007	3	.018	1	.016	4	1.738e-5	5	NC	5	NC	2
430		min	-.008	2	-.015	3	-.002	3	-9.317e-5	1	1092.3	1	4176.289	4
431	7	max	.007	3	.023	1	.021	4	1.881e-5	5	NC	5	NC	2
432		min	-.008	2	-.019	3	-.002	3	-1.85e-4	1	984.007	1	2999.449	4
433	8	max	.007	3	.026	1	.026	4	2.023e-5	5	NC	5	NC	1
434		min	-.008	2	-.021	3	-.003	3	-2.768e-4	1	918.363	1	2266.258	4
435	9	max	.007	3	.028	1	.031	5	2.166e-5	5	NC	5	NC	1
436		min	-.008	2	-.023	3	-.004	1	-3.686e-4	1	882.357	1	1779.083	4
437	10	max	.007	3	.029	2	.038	5	2.309e-5	5	NC	5	NC	1
438		min	-.008	2	-.023	3	-.007	1	-4.604e-4	1	869.983	1	1438.929	4
439	11	max	.007	3	.028	2	.046	5	2.451e-5	5	NC	5	NC	2
440		min	-.008	2	-.022	3	-.01	1	-5.522e-4	1	874.386	2	1191.941	4
441	12	max	.007	3	.026	2	.053	5	2.594e-5	5	NC	5	NC	2







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.22	3	.122	1	6.809e-3	2	NC	15	NC	3
500		min	-.112	4	-.533	1	.006	10	-4.218e-3	3	355.349	1	1427.582	1
501	4	max	.002	1	.278	3	.184	1	7.991e-3	2	NC	15	NC	3
502		min	-.112	4	-.675	1	.011	10	-4.92e-3	3	277.561	1	957.05	1
503	5	max	.002	1	.295	3	.214	1	9.173e-3	2	NC	15	NC	10
504		min	-.112	4	-.715	1	.013	10	-5.623e-3	3	261.436	1	823.717	1
505	6	max	.002	1	.274	3	.204	1	1.035e-2	2	NC	15	NC	10
506		min	-.112	4	-.655	1	.01	10	-6.325e-3	3	286.38	1	863.387	1
507	7	max	.002	1	.22	3	.156	1	1.154e-2	2	NC	5	NC	5
508		min	-.112	4	-.515	1	.004	10	-7.027e-3	3	368.231	1	1122.36	1
509	8	max	.002	1	.151	3	.083	1	1.272e-2	2	NC	5	NC	5
510		min	-.112	4	-.334	1	-.004	10	-7.729e-3	3	584.706	1	2045.812	1
511	9	max	.003	1	.086	3	.024	3	1.39e-2	2	NC	5	NC	1
512		min	-.112	4	-.169	1	-.014	2	-8.432e-3	3	1264.742	1	NC	1
513	10	max	.003	1	.057	3	.022	3	1.508e-2	2	NC	4	NC	1
514		min	-.112	4	-.095	2	-.028	2	-9.134e-3	3	2678.328	1	8797.012	2
515	11	max	.003	1	.086	3	.022	3	1.39e-2	2	NC	5	NC	1
516		min	-.112	4	-.169	1	-.014	2	-8.431e-3	3	1264.742	1	NC	1
517	12	max	.003	1	.151	3	.081	1	1.272e-2	2	NC	5	NC	3
518		min	-.112	4	-.334	1	-.004	10	-7.728e-3	3	584.706	1	2095.539	1
519	13	max	.003	1	.22	3	.153	1	1.154e-2	2	NC	5	NC	5
520		min	-.112	4	-.515	1	.004	10	-7.025e-3	3	368.231	1	1143.77	1
521	14	max	.003	1	.274	3	.2	1	1.036e-2	2	NC	15	NC	5
522		min	-.112	4	-.655	1	.006	15	-6.323e-3	3	286.381	1	878.939	1
523	15	max	.003	1	.295	3	.21	1	9.174e-3	2	NC	15	NC	3
524		min	-.112	4	-.715	1	0	15	-5.62e-3	3	261.436	1	839.24	1
525	16	max	.003	1	.278	3	.18	1	7.992e-3	2	NC	15	NC	3
526		min	-.112	4	-.675	1	-.006	5	-4.917e-3	3	277.562	1	977.609	1
527	17	max	.003	1	.22	3	.118	1	6.811e-3	2	NC	15	NC	3
528		min	-.112	4	-.533	1	-.012	5	-4.214e-3	3	355.349	1	1466.231	1
529	18	max	.003	1	.129	3	.046	1	5.629e-3	2	NC	5	NC	3
530		min	-.112	4	-.305	1	-.011	5	-3.511e-3	3	645.445	1	3543.049	1
531	19	max	.003	1	.018	3	.007	3	4.448e-3	2	NC	1	NC	1
532		min	-.112	4	-.028	2	-.008	2	-2.808e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.393e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.139e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.012	4	8.521e-4	3	NC	5	NC	1
536		min	0	5	-.015	1	0	3	-6.651e-4	1	6426.174	1	8251.529	4
537	3	max	0	3	-.002	15	.025	4	1.365e-3	3	NC	5	NC	1
538		min	-.002	5	-.03	1	-.003	3	-1.296e-3	1	3270.061	1	3819.19	4
539	4	max	0	3	-.003	15	.039	4	1.878e-3	3	NC	5	NC	9
540		min	-.003	5	-.044	1	-.007	3	-1.927e-3	1	2243.453	1	2462.156	4
541	5	max	0	3	-.003	15	.052	4	2.391e-3	3	NC	5	NC	9
542		min	-.004	5	-.056	1	-.011	3	-2.557e-3	1	1750.589	1	1848.9	4
543	6	max	0	3	-.004	15	.063	4	2.903e-3	3	NC	5	8591.203	9
544		min	-.005	5	-.067	1	-.016	3	-3.188e-3	1	1473.306	1	1526.164	4
545	7	max	0	3	-.005	15	.071	4	3.416e-3	3	NC	15	6753.318	9
546		min	-.006	5	-.075	1	-.022	3	-3.819e-3	1	1306.556	1	1349.419	4
547	8	max	0	3	-.005	15	.076	4	3.929e-3	3	NC	15	5591.912	9
548		min	-.006	5	-.082	1	-.027	3	-4.449e-3	1	1206.482	1	1261.61	4
549	9	max	0	3	-.005	15	.078	4	4.442e-3	3	NC	15	4829.377	9
550		min	-.007	5	-.085	1	-.031	3	-5.08e-3	1	1152.615	1	1239.453	4
551	10	max	0	3	-.005	15	.075	4	4.955e-3	3	NC	15	4325.462	9
552		min	-.008	5	-.087	1	-.035	3	-5.711e-3	1	1135.575	1	1277.257	4
553	11	max	0	3	-.004	15	.069	4	5.467e-3	3	NC	15	4006.348	9
554		min	-.009	5	-.086	1	-.037	3	-6.342e-3	1	1152.615	1	1383.348	4
555	12	max	0	3	-.004	15	.06	4	5.98e-3	3	NC	15	3835.604	9



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.01	5	-.082	1	-.038	3	-6.972e-3	1	1206.482	1	1584.572	4
557		max	0	3	-.003	15	.049	4	6.493e-3	3	NC	15	3803.328	9
558		min	-.011	5	-.076	1	-.037	3	-7.603e-3	1	1306.556	1	1637.785	1
559		max	0	3	-.002	15	.039	1	7.006e-3	3	NC	5	5045.582	15
560		min	-.012	5	-.068	1	-.034	3	-8.234e-3	1	1473.306	1	1690.501	1
561		max	0	3	-.001	15	.033	1	7.519e-3	3	NC	5	8899.029	15
562		min	-.013	5	-.058	1	-.029	3	-8.865e-3	1	1750.589	1	1836.972	1
563		max	0	3	0	15	.024	1	8.031e-3	3	NC	5	NC	5
564		min	-.014	5	-.046	1	-.02	3	-9.495e-3	1	2243.453	1	2148.88	1
565		max	0	3	.001	5	.011	1	8.544e-3	3	NC	5	NC	4
566		min	-.015	5	-.032	1	-.008	3	-1.013e-2	1	3270.061	1	2850.851	1
567		max	.001	3	.004	5	.007	3	9.057e-3	3	NC	5	NC	4
568		min	-.016	5	-.018	1	-.011	2	-1.076e-2	1	6426.174	2	5078.884	1
569		max	.001	3	.006	5	.026	3	9.57e-3	3	NC	1	NC	1
570		min	-.017	5	-.003	9	-.03	2	-1.139e-2	1	NC	1	NC	1
571	M16A	max	0	10	0	3	.008	3	2.851e-3	3	NC	1	NC	1
572		min	-.006	4	-.004	4	-.008	2	-2.895e-3	2	NC	1	NC	1
573		max	0	10	-.007	12	.005	1	2.732e-3	3	NC	12	NC	2
574		min	-.006	4	-.028	4	-.003	5	-2.766e-3	2	3982.689	4	9452.216	1
575		max	0	10	-.015	12	.013	1	2.613e-3	3	6540.121	12	NC	4
576		min	-.006	4	-.051	4	-.011	5	-2.637e-3	2	2026.654	4	5346.673	1
577		max	0	10	-.021	12	.019	1	2.494e-3	3	4486.905	12	NC	10
578		min	-.005	4	-.073	4	-.023	5	-2.509e-3	2	1390.403	4	4065.344	1
579		max	0	10	-.028	12	.023	1	2.375e-3	3	3501.178	12	NC	10
580		min	-.005	4	-.092	4	-.037	5	-2.38e-3	2	1084.946	4	2696.347	5
581		max	0	10	-.033	12	.026	1	2.256e-3	3	2946.611	12	NC	10
582		min	-.005	4	-.109	4	-.052	5	-2.252e-3	2	913.097	4	1915.095	5
583		max	0	10	-.037	12	.027	1	2.137e-3	3	2613.112	12	NC	10
584		min	-.004	4	-.122	4	-.065	5	-2.123e-3	2	809.752	4	1504.176	5
585		max	0	10	-.04	12	.026	1	2.018e-3	3	2412.963	12	NC	10
586		min	-.004	4	-.132	4	-.077	5	-1.995e-3	2	747.73	4	1270.328	5
587		max	0	10	-.042	12	.025	1	1.899e-3	3	2305.231	12	NC	10
588		min	-.003	4	-.138	4	-.086	5	-1.866e-3	2	714.346	4	1136.08	5
589		max	0	10	-.043	12	.022	1	1.78e-3	3	2271.15	12	NC	10
590		min	-.003	4	-.14	4	-.092	5	-1.737e-3	2	703.784	4	1066.883	5
591		max	0	10	-.042	12	.019	1	1.661e-3	3	2305.231	12	NC	10
592		min	-.003	4	-.138	4	-.093	5	-1.609e-3	2	714.346	4	1047.752	5
593		max	0	10	-.04	12	.016	1	1.542e-3	3	2412.963	12	NC	10
594		min	-.002	4	-.131	4	-.091	5	-1.48e-3	2	747.73	4	1075.221	5
595		max	0	10	-.037	12	.013	1	1.423e-3	3	2613.112	12	NC	3
596		min	-.002	4	-.121	4	-.084	5	-1.352e-3	2	809.752	4	1155.846	5
597		max	0	10	-.033	12	.009	1	1.304e-3	3	2946.611	12	NC	2
598		min	-.002	4	-.107	4	-.074	5	-1.223e-3	2	913.097	4	1310.049	5
599		max	0	10	-.028	12	.006	1	1.185e-3	3	3501.178	12	NC	1
600		min	-.001	4	-.09	4	-.062	5	-1.094e-3	2	1084.946	4	1585.826	5
601		max	0	10	-.022	12	.003	1	1.066e-3	3	4486.905	12	NC	1
602		min	-.001	4	-.07	4	-.046	5	-9.659e-4	2	1390.403	4	2102.986	5
603		max	0	10	-.015	12	.001	1	9.472e-4	3	6540.121	12	NC	1
604		min	0	4	-.048	4	-.03	5	-8.373e-4	2	2026.654	4	3233.956	5
605		max	0	10	-.008	12	0	3	9.336e-4	4	NC	12	NC	1
606		min	0	4	-.025	4	-.014	5	-7.087e-4	2	3982.689	4	6880.926	5
607		max	0	1	0	1	0	1	1.008e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.802e-4	2	NC	1	NC	1



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Address:			
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E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

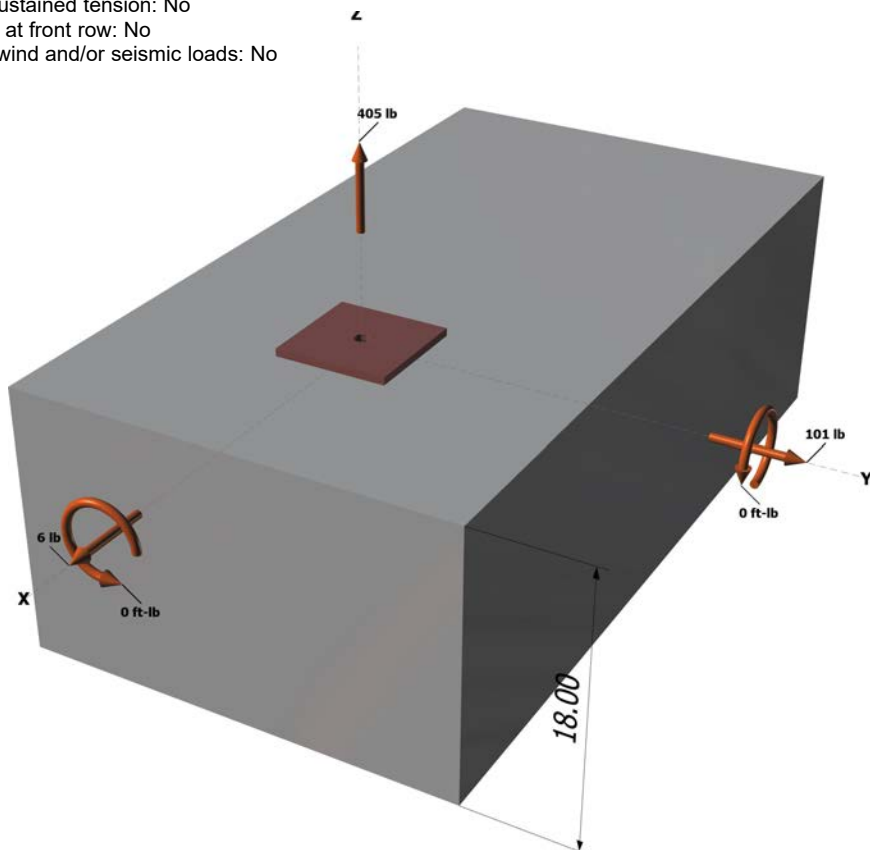
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

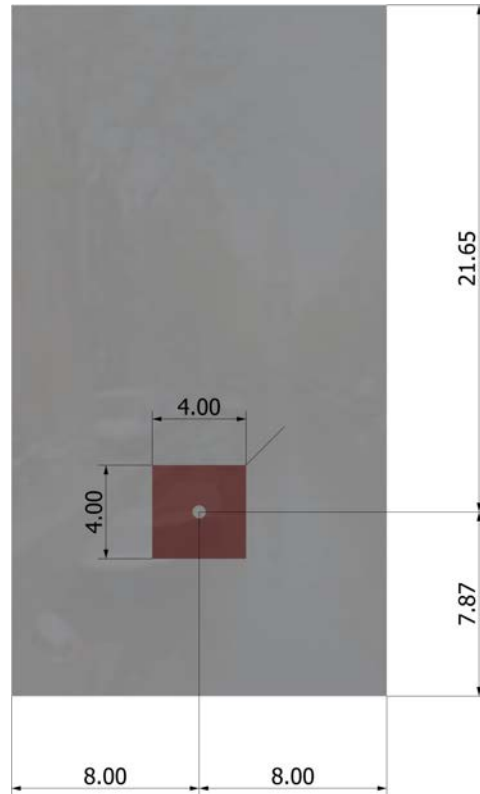
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

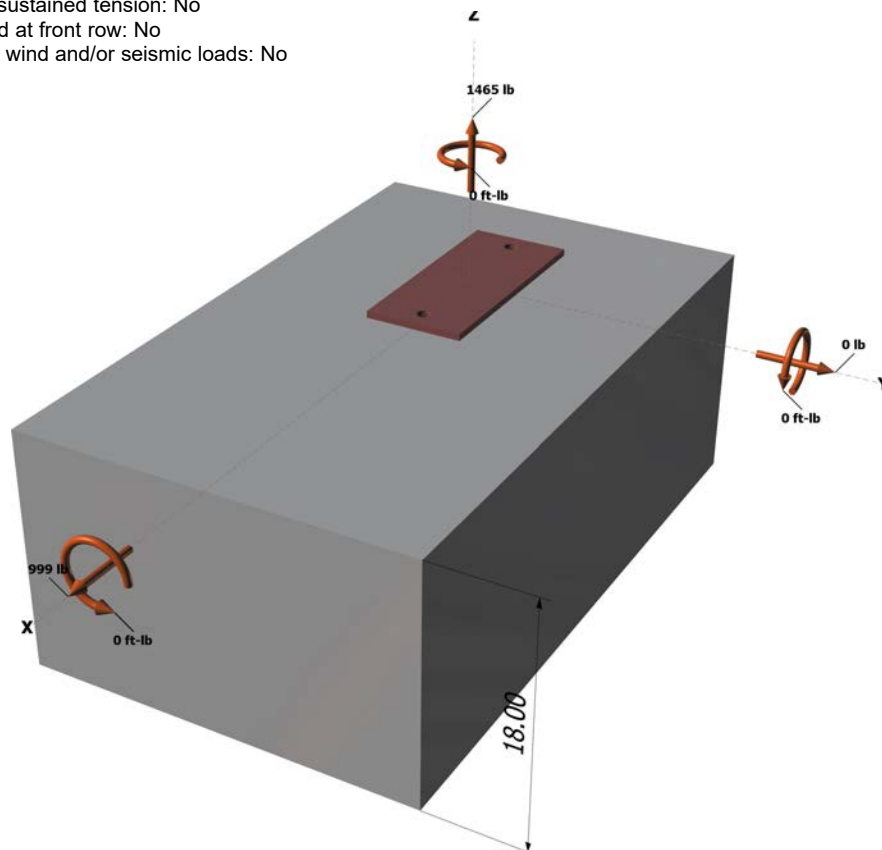
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

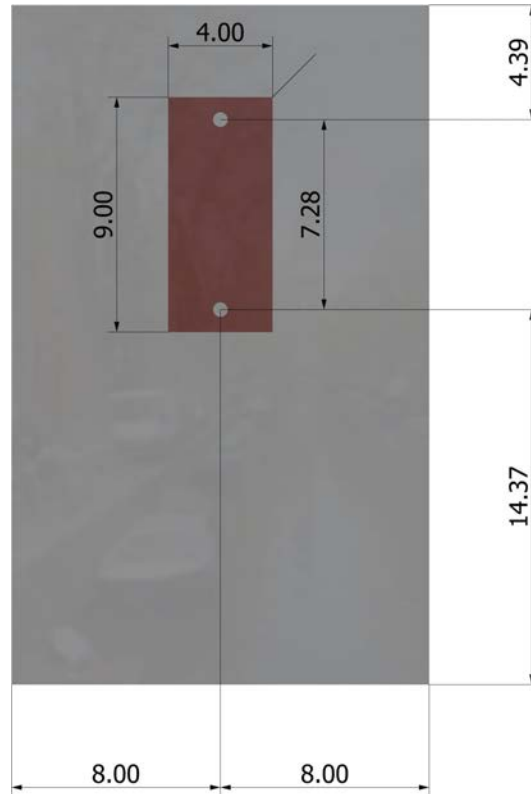
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

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Engineer:	HCV	Page:	4/5
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Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.