

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	85 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 11.34 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	126 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.894 k-ft
$M_z$ =	0.331 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>97%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.887 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.182 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>84%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.245 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>12%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.200 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>17%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	55.91 in
$\Phi F_{ty \text{ AXIAL}}$ =	15.92 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.294 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	15.642 k
Utilization =	<u>22%</u>



DETAIL VIEW

### 5. FOUNDATION DESIGN CALCULATIONS

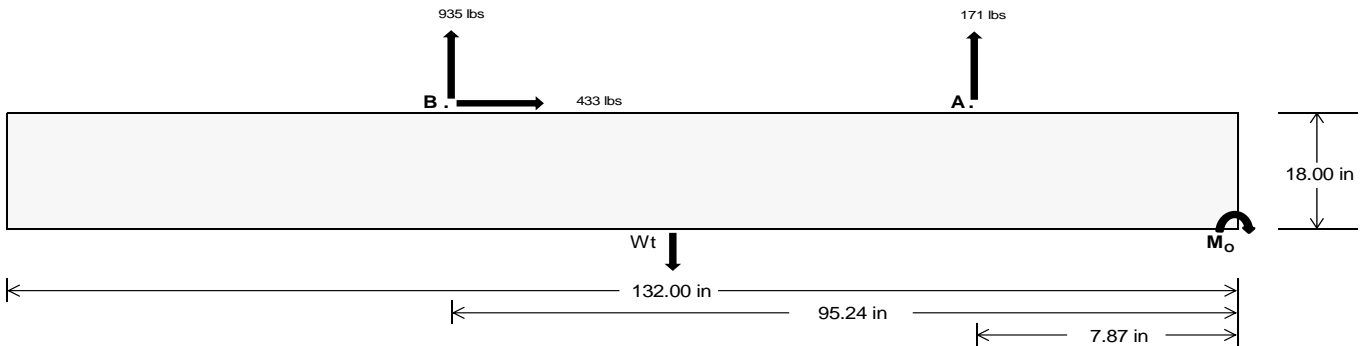
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =		<u>722.78</u>	<u>3902.09</u> k
Compressive Load =		<u>4218.36</u>	<u>4527.40</u> k
Lateral Load =		<u>13.52</u>	<u>1803.09</u> k
Moment (Weak Axis) =		<u>0.03</u>	<u>0.01</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 98169.5$  in-lbs  
Resisting Force Required = 1487.42 lbs  
S.F. = 1.67  
Weight Required = 2479.03 lbs  
Minimum Width = 21 in  
Weight Provided = 4186.88 lbs

### Sliding

Force = 433.16 lbs  
Friction = 0.4  
Weight Required = 1082.90 lbs  
Resisting Weight = 4186.88 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 433.16 lbs  
Cohesion = 130 psf  
Area = 19.25 ft<sup>2</sup>  
Resisting = 2093.44 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 132in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$   

Ballast Width	21 in	22 in	23 in	24 in
	4187 lbs	4386 lbs	4586 lbs	4785 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	1630 lbs	1630 lbs	1630 lbs	1630 lbs	1200 lbs	1200 lbs	1200 lbs	1200 lbs	1989 lbs	1989 lbs	1989 lbs	1989 lbs	-341 lbs	-341 lbs	-341 lbs	-341 lbs
$F_B$	1665 lbs	1665 lbs	1665 lbs	1665 lbs	1431 lbs	1431 lbs	1431 lbs	1431 lbs	2185 lbs	2185 lbs	2185 lbs	2185 lbs	-1870 lbs	-1870 lbs	-1870 lbs	-1870 lbs
$F_V$	187 lbs	187 lbs	187 lbs	187 lbs	788 lbs	788 lbs	788 lbs	788 lbs	717 lbs	717 lbs	717 lbs	717 lbs	-866 lbs	-866 lbs	-866 lbs	-866 lbs
$P_{total}$	7483 lbs	7682 lbs	7881 lbs	8081 lbs	6818 lbs	7017 lbs	7216 lbs	7416 lbs	8361 lbs	8560 lbs	8760 lbs	8959 lbs	301 lbs	421 lbs	541 lbs	660 lbs
$M$	4121 lbs-ft	4121 lbs-ft	4121 lbs-ft	4121 lbs-ft	3510 lbs-ft	3510 lbs-ft	3510 lbs-ft	3510 lbs-ft	5384 lbs-ft	5384 lbs-ft	5384 lbs-ft	5384 lbs-ft	1603 lbs-ft	1603 lbs-ft	1603 lbs-ft	1603 lbs-ft
$e$	0.55 ft	0.54 ft	0.52 ft	0.51 ft	0.51 ft	0.50 ft	0.49 ft	0.47 ft	0.64 ft	0.63 ft	0.61 ft	0.60 ft	5.32 ft	3.81 ft	2.97 ft	2.43 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	271.9 psf	269.5 psf	267.2 psf	265.1 psf	254.7 psf	253.0 psf	251.5 psf	250.1 psf	281.8 psf	278.8 psf	276.2 psf	273.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	505.5 psf	492.4 psf	480.4 psf	469.5 psf	453.6 psf	442.9 psf	433.1 psf	424.1 psf	586.9 psf	570.1 psf	554.8 psf	540.7 psf	640.2 psf	90.5 psf	74.2 psf	71.7 psf

Maximum Bearing Pressure = 640 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

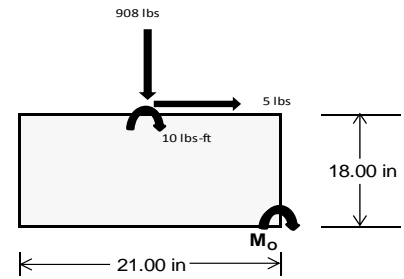
#### Overturning Check

$M_o = 776.6 \text{ ft-lbs}$   
 Resisting Force Required = 887.52 lbs  
 S.F. = 1.67  
 Weight Required = 1479.21 lbs  
 Minimum Width = 21 in  
 Weight Provided = 4186.88 lbs

*A minimum 132in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	250 lbs	671 lbs	250 lbs	908 lbs	2704 lbs	908 lbs	73 lbs	196 lbs	73 lbs
$F_v$	1 lbs	0 lbs	1 lbs	5 lbs	0 lbs	5 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	5434 lbs	4187 lbs	5434 lbs	5842 lbs	4187 lbs	5842 lbs	1589 lbs	4187 lbs	1589 lbs
$M$	5 lbs-ft	0 lbs-ft	5 lbs-ft	18 lbs-ft	0 lbs-ft	18 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft
$f_{min}$	281.4 psf	217.5 psf	281.4 psf	300.3 psf	217.5 psf	300.3 psf	82.4 psf	217.5 psf	82.4 psf
$f_{max}$	283.1 psf	217.5 psf	283.1 psf	306.6 psf	217.5 psf	306.6 psf	82.6 psf	217.5 psf	82.6 psf



Maximum Bearing Pressure = 307 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 132in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.403 k
Allowable Uplift =	1.214 k
Utilization =	<u>33%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.486 k
Allowable Uplift =	4.357 k
Utilization =	<u>34%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.245 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

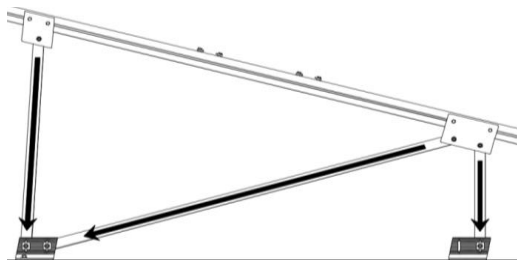
#### Rear Strut

Maximum Axial Load =	3.294 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

#### Diagonal Strut

Maximum Axial Load =	1.252 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>17%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	40.12 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.802 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 126 \text{ in}$$

$$J = 0.432$$

$$348.575$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 126$$

$$J = 0.432$$

$$221.673$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \text{ in} \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.29339$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76107$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 15.9235 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 15.92 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 16.39 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-54.031	-54.031	0	0
2	M14	Y	-54.031	-54.031	0	0
3	M15	Y	-54.031	-54.031	0	0
4	M16	Y	-54.031	-54.031	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-33.217	-33.217	0	0
2	M14	y	-33.217	-33.217	0	0
3	M15	y	-52.198	-52.198	0	0
4	M16	y	-52.198	-52.198	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	75.924	75.924	0	0
2	M14	y	58.208	58.208	0	0
3	M15	y	31.635	31.635	0	0
4	M16	y	31.635	31.635	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9			5 1.6												
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2		6 1.3												
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56			6 1.3												
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2		6 1.25												
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56			6 1.25												





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	110.463	1	229.933	1	-611	12	.012	1	-0.006	15	.694	3
28			min	4.032	15	-220.678	3	-26.166	1	0	3	-.156	1	-.682	1
29		15	max	110.463	1	91.17	1	13.056	1	.012	1	-0.006	12	.872	3
30			min	4.032	15	-84.828	3	.482	15	0	3	-.164	1	-.87	1
31		16	max	110.463	1	51.022	3	52.277	1	.012	1	-.004	12	.892	3
32			min	4.032	15	-47.593	1	1.907	15	0	3	-.125	1	-.895	1
33		17	max	110.463	1	186.872	3	91.499	1	.012	1	0	12	.753	3
34			min	4.032	15	-186.356	1	3.333	15	0	3	-.042	1	-.759	1
35		18	max	110.463	1	322.722	3	130.72	1	.012	1	.088	1	.456	3
36			min	4.032	15	-325.118	1	4.759	15	0	3	.003	15	-.46	1
37		19	max	110.463	1	458.572	3	169.942	1	.012	1	.263	1	0	1
38			min	4.032	15	-463.881	1	6.185	15	0	3	.01	15	0	3
39	M14	1	max	49.87	1	487.488	1	-6.368	15	.005	3	.299	1	0	1
40			min	1.824	15	-355.644	3	-174.996	1	-.01	1	.011	15	0	3
41		2	max	49.87	1	348.725	1	-4.943	15	.005	3	.117	1	.355	3
42			min	1.824	15	-252.864	3	-135.775	1	-.01	1	.004	15	-.488	1
43		3	max	49.87	1	209.962	1	-3.517	15	.005	3	0	3	.59	3
44			min	1.824	15	-150.084	3	-96.553	1	-.01	1	-.018	1	-.814	1
45		4	max	49.87	1	71.199	1	-2.091	15	.005	3	-.003	12	.705	3
46			min	1.824	15	-47.304	3	-57.332	1	-.01	1	-.108	1	-.978	1
47		5	max	49.87	1	55.476	3	-.665	15	.005	3	-.005	12	.7	3
48			min	1.824	15	-67.564	1	-18.11	1	-.01	1	-.152	1	-.98	1
49		6	max	49.87	1	158.256	3	21.112	1	.005	3	-.005	15	.576	3
50			min	1.824	15	-206.326	1	.433	12	-.01	1	-.15	1	-.82	1
51		7	max	49.87	1	261.036	3	60.333	1	.005	3	-.004	15	.331	3
52			min	1.824	15	-345.089	1	1.858	12	-.01	1	-.103	1	-.498	1
53		8	max	49.87	1	363.816	3	99.555	1	.005	3	0	10	0	15
54			min	1.824	15	-483.852	1	3.284	12	-.01	1	-.009	1	-.033	3
55		9	max	49.87	1	466.597	3	138.776	1	.005	3	.13	1	.631	1
56			min	1.824	15	-622.615	1	4.709	12	-.01	1	.003	12	-.518	3
57		10	max	49.87	1	569.377	3	177.998	1	.005	3	.315	1	1.438	1
58			min	1.824	15	-761.377	1	6.135	12	-.01	1	.01	12	-1.122	3
59		11	max	49.87	1	622.615	1	-4.709	12	.01	1	.13	1	.631	1
60			min	1.824	15	-466.597	3	-138.776	1	-.005	3	.003	12	-.518	3
61		12	max	49.87	1	483.852	1	-3.284	12	.01	1	0	10	0	15
62			min	1.824	15	-363.816	3	-99.555	1	-.005	3	-.009	1	-.033	3
63		13	max	49.87	1	345.089	1	-1.858	12	.01	1	-.004	15	.331	3
64			min	1.824	15	-261.036	3	-60.333	1	-.005	3	-.103	1	-.498	1
65		14	max	49.87	1	206.326	1	-.433	12	.01	1	-.005	15	.576	3
66			min	1.824	15	-158.256	3	-21.112	1	-.005	3	-.15	1	-.82	1
67		15	max	49.87	1	67.564	1	18.11	1	.01	1	-.005	12	.7	3
68			min	1.824	15	-55.476	3	.665	15	-.005	3	-.152	1	-.98	1
69		16	max	49.87	1	47.304	3	57.332	1	.01	1	-.003	12	.705	3
70			min	1.824	15	-71.199	1	2.091	15	-.005	3	-.108	1	-.978	1
71		17	max	49.87	1	150.084	3	96.553	1	.01	1	0	3	.59	3
72			min	1.824	15	-209.962	1	3.517	15	-.005	3	-.018	1	-.814	1
73		18	max	49.87	1	252.864	3	135.775	1	.01	1	.117	1	.355	3
74			min	1.824	15	-348.725	1	4.943	15	-.005	3	.004	15	-.488	1
75		19	max	49.87	1	355.644	3	174.996	1	.01	1	.299	1	0	1
76			min	1.824	15	-487.488	1	6.368	15	-.005	3	.011	15	0	3
77	M15	1	max	-1.923	15	547.256	1	-6.367	15	.01	1	.299	1	0	2
78			min	-52.569	1	-188.374	3	-174.966	1	-.004	3	.011	15	0	12
79		2	max	-1.923	15	390.777	1	-4.941	15	.01	1	.117	1	.189	3
80			min	-52.569	1	-135.196	3	-135.745	1	-.004	3	.004	15	-.547	1
81		3	max	-1.923	15	234.299	1	-3.515	15	.01	1	0	3	.315	3
82			min	-52.569	1	-82.019	3	-96.523	1	-.004	3	-.018	1	-.912	1
83		4	max	-1.923	15	77.821	1	-2.09	15	.01	1	-.003	12	.38	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84		min	-52.569	1	-28.842	3	-57.302	1	-.004	3	-.108	1	-1.094	1
85	5	max	-1.923	15	24.335	3	-.664	15	.01	1	-.005	12	.383	3
86		min	-52.569	1	-78.658	1	-18.08	1	-.004	3	-.152	1	-1.093	1
87	6	max	-1.923	15	77.512	3	21.141	1	.01	1	-.005	15	.323	3
88		min	-52.569	1	-235.136	1	.459	12	-.004	3	-.15	1	-.91	1
89	7	max	-1.923	15	130.689	3	60.363	1	.01	1	-.004	15	.202	3
90		min	-52.569	1	-391.614	1	1.884	12	-.004	3	-.103	1	-.545	1
91	8	max	-1.923	15	183.867	3	99.585	1	.01	1	0	10	.018	3
92		min	-52.569	1	-548.093	1	3.31	12	-.004	3	-.009	1	-.003	9
93	9	max	-1.923	15	237.044	3	138.806	1	.01	1	.13	1	.734	1
94		min	-52.569	1	-704.571	1	4.735	12	-.004	3	.003	12	-.227	3
95	10	max	-1.923	15	290.221	3	178.028	1	.01	1	.315	1	1.647	1
96		min	-52.569	1	-861.05	1	6.161	12	-.004	3	.01	12	-.535	3
97	11	max	-1.923	15	704.571	1	-4.735	12	.004	3	.13	1	.734	1
98		min	-52.569	1	-237.044	3	-138.806	1	-.01	1	.003	12	-.227	3
99	12	max	-1.923	15	548.093	1	-3.31	12	.004	3	0	10	.018	3
100		min	-52.569	1	-183.867	3	-99.585	1	-.01	1	-.009	1	-.003	9
101	13	max	-1.923	15	391.614	1	-1.884	12	.004	3	-.004	15	.202	3
102		min	-52.569	1	-130.689	3	-60.363	1	-.01	1	-.103	1	-.545	1
103	14	max	-1.923	15	235.136	1	-.459	12	.004	3	-.005	15	.323	3
104		min	-52.569	1	-77.512	3	-21.141	1	-.01	1	-.15	1	-.91	1
105	15	max	-1.923	15	78.658	1	18.08	1	.004	3	-.005	12	.383	3
106		min	-52.569	1	-24.335	3	.664	15	-.01	1	-.152	1	-1.093	1
107	16	max	-1.923	15	28.842	3	57.302	1	.004	3	-.003	12	.38	3
108		min	-52.569	1	-77.821	1	2.09	15	-.01	1	-.108	1	-1.094	1
109	17	max	-1.923	15	82.019	3	96.523	1	.004	3	0	3	.315	3
110		min	-52.569	1	-234.299	1	3.515	15	-.01	1	-.018	1	-.912	1
111	18	max	-1.923	15	135.196	3	135.745	1	.004	3	.117	1	.189	3
112		min	-52.569	1	-390.777	1	4.941	15	-.01	1	.004	15	-.547	1
113	19	max	-1.923	15	188.374	3	174.966	1	.004	3	.299	1	0	2
114		min	-52.569	1	-547.256	1	6.367	15	-.01	1	.011	15	0	12
115	M16	1	max	-4.285	15	523.803	1	-6.19	.011	1	.265	1	0	1
116		min	-117.213	1	-176.987	3	-170.139	1	-.007	3	.01	15	0	3
117	2	max	-4.285	15	367.324	1	-4.765	15	.011	1	.089	1	.175	3
118		min	-117.213	1	-123.809	3	-130.917	1	-.007	3	.003	15	-.52	1
119	3	max	-4.285	15	210.846	1	-3.339	15	.011	1	0	12	.289	3
120		min	-117.213	1	-70.632	3	-91.696	1	-.007	3	-.041	1	-.857	1
121	4	max	-4.285	15	54.368	1	-1.913	15	.011	1	-.004	12	.34	3
122		min	-117.213	1	-17.455	3	-52.474	1	-.007	3	-.125	1	-1.012	1
123	5	max	-4.285	15	35.722	3	-.487	15	.011	1	-.006	12	.33	3
124		min	-117.213	1	-102.111	1	-13.253	1	-.007	3	-.163	1	-.984	1
125	6	max	-4.285	15	88.899	3	25.969	1	.011	1	-.006	15	.257	3
126		min	-117.213	1	-258.589	1	.694	12	-.007	3	-.156	1	-.774	1
127	7	max	-4.285	15	142.076	3	65.19	1	.011	1	-.004	15	.122	3
128		min	-117.213	1	-415.067	1	2.12	12	-.007	3	-.103	1	-.381	1
129	8	max	-4.285	15	195.254	3	104.412	1	.011	1	0	10	.195	1
130		min	-117.213	1	-571.546	1	3.545	12	-.007	3	-.004	1	-.075	3
131	9	max	-4.285	15	248.431	3	143.633	1	.011	1	.141	1	.953	1
132		min	-117.213	1	-728.024	1	4.971	12	-.007	3	.004	12	-.333	3
133	10	max	-4.285	15	301.608	3	182.855	1	.011	1	.331	1	1.894	1
134		min	-117.213	1	-884.503	1	6.396	12	-.007	3	.011	12	-.654	3
135	11	max	-4.285	15	728.024	1	-4.971	12	.007	3	.141	1	.953	1
136		min	-117.213	1	-248.431	3	-143.633	1	-.011	1	.004	12	-.333	3
137	12	max	-4.285	15	571.546	1	-3.545	12	.007	3	0	10	.195	1
138		min	-117.213	1	-195.254	3	-104.412	1	-.011	1	-.004	1	-.075	3
139	13	max	-4.285	15	415.067	1	-2.12	12	.007	3	-.004	15	.122	3
140		min	-117.213	1	-142.076	3	-65.19	1	-.011	1	-.103	1	-.381	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-4.285	15	258.589	1	-.694	12	.007	3	-.006	15	.257	3
142			min	-117.213	1	-88.899	3	-25.969	1	-.011	1	-.156	1	-.774	1
143		15	max	-4.285	15	102.111	1	13.253	1	.007	3	-.006	12	.33	3
144			min	-117.213	1	-35.722	3	.487	15	-.011	1	-.163	1	-.984	1
145		16	max	-4.285	15	17.455	3	52.474	1	.007	3	-.004	12	.34	3
146			min	-117.213	1	-54.368	1	1.913	15	-.011	1	-.125	1	-1.012	1
147		17	max	-4.285	15	70.632	3	91.696	1	.007	3	0	12	.289	3
148			min	-117.213	1	-210.846	1	3.339	15	-.011	1	-.041	1	-.857	1
149		18	max	-4.285	15	123.809	3	130.917	1	.007	3	.089	1	.175	3
150			min	-117.213	1	-367.324	1	4.765	15	-.011	1	.003	15	-.52	1
151		19	max	-4.285	15	176.987	3	170.139	1	.007	3	.265	1	0	1
152			min	-117.213	1	-523.803	1	6.19	15	-.011	1	.01	15	0	3
153	M2	1	max	1018.373	1	2.025	4	1.029	1	0	3	0	3	0	1
154			min	-813.012	3	.477	15	.037	15	0	1	0	1	0	1
155		2	max	1018.752	1	1.992	4	1.029	1	0	3	0	1	0	15
156			min	-812.728	3	.469	15	.037	15	0	1	0	15	0	4
157		3	max	1019.132	1	1.959	4	1.029	1	0	3	0	1	0	15
158			min	-812.443	3	.462	15	.037	15	0	1	0	15	-.001	4
159		4	max	1019.511	1	1.925	4	1.029	1	0	3	0	1	0	15
160			min	-812.159	3	.454	15	.037	15	0	1	0	15	-.002	4
161		5	max	1019.89	1	1.892	4	1.029	1	0	3	.001	1	0	15
162			min	-811.874	3	.446	15	.037	15	0	1	0	15	-.002	4
163		6	max	1020.269	1	1.859	4	1.029	1	0	3	.001	1	0	15
164			min	-811.59	3	.438	15	.037	15	0	1	0	15	-.002	4
165		7	max	1020.649	1	1.825	4	1.029	1	0	3	.002	1	0	15
166			min	-811.306	3	.43	15	.037	15	0	1	0	15	-.003	4
167		8	max	1021.028	1	1.792	4	1.029	1	0	3	.002	1	0	15
168			min	-811.021	3	.422	15	.037	15	0	1	0	15	-.003	4
169		9	max	1021.407	1	1.758	4	1.029	1	0	3	.002	1	0	15
170			min	-810.737	3	.414	15	.037	15	0	1	0	15	-.004	4
171		10	max	1021.787	1	1.725	4	1.029	1	0	3	.002	1	-.001	15
172			min	-810.452	3	.407	15	.037	15	0	1	0	15	-.004	4
173		11	max	1022.166	1	1.692	4	1.029	1	0	3	.003	1	-.001	15
174			min	-810.168	3	.399	15	.037	15	0	1	0	15	-.005	4
175		12	max	1022.545	1	1.658	4	1.029	1	0	3	.003	1	-.001	15
176			min	-809.883	3	.391	15	.037	15	0	1	0	15	-.005	4
177		13	max	1022.924	1	1.625	4	1.029	1	0	3	.003	1	-.001	15
178			min	-809.599	3	.383	15	.037	15	0	1	0	15	-.006	4
179		14	max	1023.304	1	1.591	4	1.029	1	0	3	.003	1	-.001	15
180			min	-809.314	3	.375	15	.037	15	0	1	0	15	-.006	4
181		15	max	1023.683	1	1.558	4	1.029	1	0	3	.004	1	-.002	15
182			min	-809.03	3	.367	15	.037	15	0	1	0	15	-.006	4
183		16	max	1024.062	1	1.525	4	1.029	1	0	3	.004	1	-.002	15
184			min	-808.746	3	.36	15	.037	15	0	1	0	15	-.007	4
185		17	max	1024.441	1	1.491	4	1.029	1	0	3	.004	1	-.002	15
186			min	-808.461	3	.352	15	.037	15	0	1	0	15	-.007	4
187		18	max	1024.821	1	1.458	4	1.029	1	0	3	.004	1	-.002	15
188			min	-808.177	3	.344	15	.037	15	0	1	0	15	-.008	4
189		19	max	1025.2	1	1.424	4	1.029	1	0	3	.005	1	-.002	15
190			min	-807.892	3	.336	15	.037	15	0	1	0	15	-.008	4
191	M3	1	max	274.349	2	7.981	4	.081	1	0	3	0	1	.008	4
192			min	-399.792	3	1.877	15	.003	15	0	1	0	15	.002	15
193		2	max	274.179	2	7.211	4	.081	1	0	3	0	1	.005	4
194			min	-399.92	3	1.696	15	.003	15	0	1	0	15	.001	15
195		3	max	274.009	2	6.441	4	.081	1	0	3	0	1	.002	2
196			min	-400.047	3	1.515	15	.003	15	0	1	0	15	0	12
197		4	max	273.838	2	5.671	4	.081	1	0	3	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-400.175	3	1.334	15	.003	15	0	1	0	15	-.001	3
199	5	max	273.668	2	4.901	4	.081	1	0	3	0	1	0	15
200		min	-400.303	3	1.153	15	.003	15	0	1	0	15	-.003	4
201	6	max	273.498	2	4.131	4	.081	1	0	3	0	1	-.001	15
202		min	-400.431	3	.972	15	.003	15	0	1	0	15	-.005	4
203	7	max	273.327	2	3.361	4	.081	1	0	3	0	1	-.001	15
204		min	-400.558	3	.791	15	.003	15	0	1	0	15	-.006	4
205	8	max	273.157	2	2.591	4	.081	1	0	3	0	1	-.002	15
206		min	-400.686	3	.61	15	.003	15	0	1	0	15	-.008	4
207	9	max	272.987	2	1.821	4	.081	1	0	3	0	1	-.002	15
208		min	-400.814	3	.429	15	.003	15	0	1	0	15	-.009	4
209	10	max	272.816	2	1.051	4	.081	1	0	3	0	1	-.002	15
210		min	-400.942	3	.248	15	.003	15	0	1	0	15	-.009	4
211	11	max	272.646	2	.324	2	.081	1	0	3	0	1	-.002	15
212		min	-401.069	3	.019	12	.003	15	0	1	0	15	-.009	4
213	12	max	272.476	2	-.114	15	.081	1	0	3	0	1	-.002	15
214		min	-401.197	3	-.489	4	.003	15	0	1	0	15	-.009	4
215	13	max	272.305	2	-.295	15	.081	1	0	3	0	1	-.002	15
216		min	-401.325	3	-1.259	4	.003	15	0	1	0	15	-.009	4
217	14	max	272.135	2	-.476	15	.081	1	0	3	0	1	-.002	15
218		min	-401.453	3	-2.029	4	.003	15	0	1	0	15	-.008	4
219	15	max	271.965	2	-.657	15	.081	1	0	3	0	1	-.002	15
220		min	-401.581	3	-2.799	4	.003	15	0	1	0	15	-.007	4
221	16	max	271.794	2	-.838	15	.081	1	0	3	0	1	-.001	15
222		min	-401.708	3	-3.569	4	.003	15	0	1	0	15	-.006	4
223	17	max	271.624	2	-1.019	15	.081	1	0	3	0	1	-.001	15
224		min	-401.836	3	-4.339	4	.003	15	0	1	0	15	-.004	4
225	18	max	271.453	2	-1.2	15	.081	1	0	3	0	1	0	15
226		min	-401.964	3	-5.109	4	.003	15	0	1	0	15	-.002	4
227	19	max	271.283	2	-1.381	15	.081	1	0	3	0	1	0	1
228		min	-402.092	3	-5.879	4	.003	15	0	1	0	15	0	1
229	M4	1	max	1159.096	1	0	1	-.392	15	0	1	0	1	0
230		min	-151.133	3	0	1	-10.779	1	0	1	0	15	0	1
231	2	max	1159.267	1	0	1	-.392	15	0	1	0	12	0	1
232		min	-151.006	3	0	1	-10.779	1	0	1	0	1	0	1
233	3	max	1159.437	1	0	1	-.392	15	0	1	0	15	0	1
234		min	-150.878	3	0	1	-10.779	1	0	1	-.002	1	0	1
235	4	max	1159.607	1	0	1	-.392	15	0	1	0	15	0	1
236		min	-150.75	3	0	1	-10.779	1	0	1	-.003	1	0	1
237	5	max	1159.778	1	0	1	-.392	15	0	1	0	15	0	1
238		min	-150.622	3	0	1	-10.779	1	0	1	-.004	1	0	1
239	6	max	1159.948	1	0	1	-.392	15	0	1	0	15	0	1
240		min	-150.495	3	0	1	-10.779	1	0	1	-.006	1	0	1
241	7	max	1160.118	1	0	1	-.392	15	0	1	0	15	0	1
242		min	-150.367	3	0	1	-10.779	1	0	1	-.007	1	0	1
243	8	max	1160.289	1	0	1	-.392	15	0	1	0	15	0	1
244		min	-150.239	3	0	1	-10.779	1	0	1	-.008	1	0	1
245	9	max	1160.459	1	0	1	-.392	15	0	1	0	15	0	1
246		min	-150.111	3	0	1	-10.779	1	0	1	-.009	1	0	1
247	10	max	1160.629	1	0	1	-.392	15	0	1	0	15	0	1
248		min	-149.984	3	0	1	-10.779	1	0	1	-.011	1	0	1
249	11	max	1160.8	1	0	1	-.392	15	0	1	0	15	0	1
250		min	-149.856	3	0	1	-10.779	1	0	1	-.012	1	0	1
251	12	max	1160.97	1	0	1	-.392	15	0	1	0	15	0	1
252		min	-149.728	3	0	1	-10.779	1	0	1	-.013	1	0	1
253	13	max	1161.14	1	0	1	-.392	15	0	1	0	15	0	1
254		min	-149.6	3	0	1	-10.779	1	0	1	-.014	1	0	1





Company : Schletter, Inc.  
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Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1161.311	1	0	1	-.392	15	0	1	0	15	0	1
256		min	-149.473	3	0	1	-10.779	1	0	1	-.016	1	0	1
257	15	max	1161.481	1	0	1	-.392	15	0	1	0	15	0	1
258		min	-149.345	3	0	1	-10.779	1	0	1	-.017	1	0	1
259	16	max	1161.652	1	0	1	-.392	15	0	1	0	15	0	1
260		min	-149.217	3	0	1	-10.779	1	0	1	-.018	1	0	1
261	17	max	1161.822	1	0	1	-.392	15	0	1	0	15	0	1
262		min	-149.089	3	0	1	-10.779	1	0	1	-.019	1	0	1
263	18	max	1161.992	1	0	1	-.392	15	0	1	0	15	0	1
264		min	-148.962	3	0	1	-10.779	1	0	1	-.02	1	0	1
265	19	max	1162.163	1	0	1	-.392	15	0	1	0	15	0	1
266		min	-148.834	3	0	1	-10.779	1	0	1	-.022	1	0	1
267	M6	1	max	3286.947	1	2.255	2	0	1	0	0	1	0	1
268		min	-2675.782	3	.309	12	0	1	0	1	0	1	0	1
269	2	max	3287.326	1	2.229	2	0	1	0	1	0	1	0	12
270		min	-2675.497	3	.296	12	0	1	0	1	0	1	0	2
271	3	max	3287.705	1	2.203	2	0	1	0	1	0	1	0	12
272		min	-2675.213	3	.283	12	0	1	0	1	0	1	-.001	2
273	4	max	3288.084	1	2.177	2	0	1	0	1	0	1	0	12
274		min	-2674.928	3	.27	12	0	1	0	1	0	1	-.002	2
275	5	max	3288.464	1	2.151	2	0	1	0	1	0	1	0	12
276		min	-2674.644	3	.257	12	0	1	0	1	0	1	-.002	2
277	6	max	3288.843	1	2.125	2	0	1	0	1	0	1	0	12
278		min	-2674.359	3	.244	12	0	1	0	1	0	1	-.003	2
279	7	max	3289.222	1	2.099	2	0	1	0	1	0	1	0	12
280		min	-2674.075	3	.231	12	0	1	0	1	0	1	-.003	2
281	8	max	3289.601	1	2.073	2	0	1	0	1	0	1	0	12
282		min	-2673.791	3	.218	12	0	1	0	1	0	1	-.004	2
283	9	max	3289.981	1	2.047	2	0	1	0	1	0	1	0	12
284		min	-2673.506	3	.205	12	0	1	0	1	0	1	-.004	2
285	10	max	3290.36	1	2.021	2	0	1	0	1	0	1	0	12
286		min	-2673.222	3	.192	12	0	1	0	1	0	1	-.005	2
287	11	max	3290.739	1	1.995	2	0	1	0	1	0	1	0	12
288		min	-2672.937	3	.179	12	0	1	0	1	0	1	-.005	2
289	12	max	3291.118	1	1.969	2	0	1	0	1	0	1	0	12
290		min	-2672.653	3	.166	12	0	1	0	1	0	1	-.006	2
291	13	max	3291.498	1	1.943	2	0	1	0	1	0	1	0	12
292		min	-2672.368	3	.153	12	0	1	0	1	0	1	-.006	2
293	14	max	3291.877	1	1.917	2	0	1	0	1	0	1	0	12
294		min	-2672.084	3	.14	12	0	1	0	1	0	1	-.007	2
295	15	max	3292.256	1	1.891	2	0	1	0	1	0	1	0	12
296		min	-2671.799	3	.127	12	0	1	0	1	0	1	-.007	2
297	16	max	3292.636	1	1.865	2	0	1	0	1	0	1	0	12
298		min	-2671.515	3	.114	12	0	1	0	1	0	1	-.008	2
299	17	max	3293.015	1	1.839	2	0	1	0	1	0	1	0	12
300		min	-2671.231	3	.101	12	0	1	0	1	0	1	-.008	2
301	18	max	3293.394	1	1.813	2	0	1	0	1	0	1	0	12
302		min	-2670.946	3	.083	3	0	1	0	1	0	1	-.009	2
303	19	max	3293.773	1	1.787	2	0	1	0	1	0	1	0	12
304		min	-2670.662	3	.064	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	1200.124	2	8.022	4	0	1	0	0	1	.009	2
306		min	-1249.738	3	1.882	15	0	1	0	1	0	1	0	12
307	2	max	1199.954	2	7.252	4	0	1	0	1	0	1	.007	2
308		min	-1249.866	3	1.701	15	0	1	0	1	0	1	0	3
309	3	max	1199.783	2	6.482	4	0	1	0	1	0	1	.004	2
310		min	-1249.993	3	1.52	15	0	1	0	1	0	1	-.002	3
311	4	max	1199.613	2	5.712	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-1250.121	3	1.339	15	0	1	0	1	0	1	-.003	3
313	5	max	1199.443	2	4.942	4	0	1	0	1	0	1	0	2
314		min	-1250.249	3	1.158	15	0	1	0	1	0	1	-.004	3
315	6	max	1199.272	2	4.172	4	0	1	0	1	0	1	-.001	15
316		min	-1250.377	3	.977	15	0	1	0	1	0	1	-.005	3
317	7	max	1199.102	2	3.402	4	0	1	0	1	0	1	-.001	15
318		min	-1250.504	3	.796	15	0	1	0	1	0	1	-.006	4
319	8	max	1198.932	2	2.632	4	0	1	0	1	0	1	-.002	15
320		min	-1250.632	3	.615	15	0	1	0	1	0	1	-.007	4
321	9	max	1198.761	2	1.862	4	0	1	0	1	0	1	-.002	15
322		min	-1250.76	3	.417	12	0	1	0	1	0	1	-.008	4
323	10	max	1198.591	2	1.232	2	0	1	0	1	0	1	-.002	15
324		min	-1250.888	3	.117	12	0	1	0	1	0	1	-.009	4
325	11	max	1198.421	2	.632	2	0	1	0	1	0	1	-.002	15
326		min	-1251.015	3	-.304	3	0	1	0	1	0	1	-.009	4
327	12	max	1198.25	2	.032	2	0	1	0	1	0	1	-.002	15
328		min	-1251.143	3	-.754	3	0	1	0	1	0	1	-.009	4
329	13	max	1198.08	2	-.29	15	0	1	0	1	0	1	-.002	15
330		min	-1251.271	3	-1.218	4	0	1	0	1	0	1	-.009	4
331	14	max	1197.91	2	-.471	15	0	1	0	1	0	1	-.002	15
332		min	-1251.399	3	-1.988	4	0	1	0	1	0	1	-.008	4
333	15	max	1197.739	2	-.652	15	0	1	0	1	0	1	-.002	15
334		min	-1251.527	3	-2.758	4	0	1	0	1	0	1	-.007	4
335	16	max	1197.569	2	-.833	15	0	1	0	1	0	1	-.001	15
336		min	-1251.654	3	-3.528	4	0	1	0	1	0	1	-.006	4
337	17	max	1197.399	2	-1.014	15	0	1	0	1	0	1	-.001	15
338		min	-1251.782	3	-4.298	4	0	1	0	1	0	1	-.004	4
339	18	max	1197.228	2	-1.195	15	0	1	0	1	0	1	0	15
340		min	-1251.91	3	-5.068	4	0	1	0	1	0	1	-.002	4
341	19	max	1197.058	2	-1.376	15	0	1	0	1	0	1	0	1
342		min	-1252.038	3	-5.838	4	0	1	0	1	0	1	0	1
343	M8	1	max	3241.829	1	0	1	0	1	0	1	0	1	1
344		min	-558.283	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3241.999	1	0	1	0	1	0	1	0	1	0	1
346		min	-558.156	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3242.169	1	0	1	0	1	0	1	0	1	0	1
348		min	-558.028	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3242.34	1	0	1	0	1	0	1	0	1	0	1
350		min	-557.9	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3242.51	1	0	1	0	1	0	1	0	1	0	1
352		min	-557.772	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3242.68	1	0	1	0	1	0	1	0	1	0	1
354		min	-557.645	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3242.851	1	0	1	0	1	0	1	0	1	0	1
356		min	-557.517	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3243.021	1	0	1	0	1	0	1	0	1	0	1
358		min	-557.389	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3243.191	1	0	1	0	1	0	1	0	1	0	1
360		min	-557.261	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3243.362	1	0	1	0	1	0	1	0	1	0	1
362		min	-557.134	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3243.532	1	0	1	0	1	0	1	0	1	0	1
364		min	-557.006	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3243.702	1	0	1	0	1	0	1	0	1	0	1
366		min	-556.878	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3243.873	1	0	1	0	1	0	1	0	1	0	1
368		min	-556.75	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3244.043	1	0	1	0	1	0	1	0	1	0	1
370			min	-556.623	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3244.213	1	0	1	0	1	0	1	0	1	0	1
372			min	-556.495	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3244.384	1	0	1	0	1	0	1	0	1	0	1
374			min	-556.367	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3244.554	1	0	1	0	1	0	1	0	1	0	1
376			min	-556.239	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3244.724	1	0	1	0	1	0	1	0	1	0	1
378			min	-556.112	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3244.895	1	0	1	0	1	0	1	0	1	0	1
380			min	-555.984	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1018.373	1	2.025	4	-.037	15	0	1	0	1	0	1
382			min	-813.012	3	.477	15	-1.029	1	0	3	0	3	0	1
383		2	max	1018.752	1	1.992	4	-.037	15	0	1	0	15	0	15
384			min	-812.728	3	.469	15	-1.029	1	0	3	0	1	0	4
385		3	max	1019.132	1	1.959	4	-.037	15	0	1	0	15	0	15
386			min	-812.443	3	.462	15	-1.029	1	0	3	0	1	-.001	4
387		4	max	1019.511	1	1.925	4	-.037	15	0	1	0	15	0	15
388			min	-812.159	3	.454	15	-1.029	1	0	3	0	1	-.002	4
389		5	max	1019.89	1	1.892	4	-.037	15	0	1	0	15	0	15
390			min	-811.874	3	.446	15	-1.029	1	0	3	-.001	1	-.002	4
391		6	max	1020.269	1	1.859	4	-.037	15	0	1	0	15	0	15
392			min	-811.59	3	.438	15	-1.029	1	0	3	-.001	1	-.002	4
393		7	max	1020.649	1	1.825	4	-.037	15	0	1	0	15	0	15
394			min	-811.306	3	.43	15	-1.029	1	0	3	-.002	1	-.003	4
395		8	max	1021.028	1	1.792	4	-.037	15	0	1	0	15	0	15
396			min	-811.021	3	.422	15	-1.029	1	0	3	-.002	1	-.003	4
397		9	max	1021.407	1	1.758	4	-.037	15	0	1	0	15	0	15
398			min	-810.737	3	.414	15	-1.029	1	0	3	-.002	1	-.004	4
399		10	max	1021.787	1	1.725	4	-.037	15	0	1	0	15	-.001	15
400			min	-810.452	3	.407	15	-1.029	1	0	3	-.002	1	-.004	4
401		11	max	1022.166	1	1.692	4	-.037	15	0	1	0	15	-.001	15
402			min	-810.168	3	.399	15	-1.029	1	0	3	-.003	1	-.005	4
403		12	max	1022.545	1	1.658	4	-.037	15	0	1	0	15	-.001	15
404			min	-809.883	3	.391	15	-1.029	1	0	3	-.003	1	-.005	4
405		13	max	1022.924	1	1.625	4	-.037	15	0	1	0	15	-.001	15
406			min	-809.599	3	.383	15	-1.029	1	0	3	-.003	1	-.006	4
407		14	max	1023.304	1	1.591	4	-.037	15	0	1	0	15	-.001	15
408			min	-809.314	3	.375	15	-1.029	1	0	3	-.003	1	-.006	4
409		15	max	1023.683	1	1.558	4	-.037	15	0	1	0	15	-.002	15
410			min	-809.03	3	.367	15	-1.029	1	0	3	-.004	1	-.006	4
411		16	max	1024.062	1	1.525	4	-.037	15	0	1	0	15	-.002	15
412			min	-808.746	3	.36	15	-1.029	1	0	3	-.004	1	-.007	4
413		17	max	1024.441	1	1.491	4	-.037	15	0	1	0	15	-.002	15
414			min	-808.461	3	.352	15	-1.029	1	0	3	-.004	1	-.007	4
415		18	max	1024.821	1	1.458	4	-.037	15	0	1	0	15	-.002	15
416			min	-808.177	3	.344	15	-1.029	1	0	3	-.004	1	-.008	4
417		19	max	1025.2	1	1.424	4	-.037	15	0	1	0	15	-.002	15
418			min	-807.892	3	.336	15	-1.029	1	0	3	-.005	1	-.008	4
419	M11	1	max	274.349	2	7.981	4	-.003	15	0	1	0	15	.008	4
420			min	-399.792	3	1.877	15	-.081	1	0	3	0	1	.002	15
421		2	max	274.179	2	7.211	4	-.003	15	0	1	0	15	.005	4
422			min	-399.92	3	1.696	15	-.081	1	0	3	0	1	.001	15
423		3	max	274.009	2	6.441	4	-.003	15	0	1	0	15	.002	2
424			min	-400.047	3	1.515	15	-.081	1	0	3	0	1	0	12
425		4	max	273.838	2	5.671	4	-.003	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-400.175	3	1.334	15	-.081	1	0	3	0	1	-.001	3
427		5	max	273.668	2	4.901	4	-.003	15	0	1	0	15	0	15
428			min	-400.303	3	1.153	15	-.081	1	0	3	0	1	-.003	4
429		6	max	273.498	2	4.131	4	-.003	15	0	1	0	15	-.001	15
430			min	-400.431	3	.972	15	-.081	1	0	3	0	1	-.005	4
431		7	max	273.327	2	3.361	4	-.003	15	0	1	0	15	-.001	15
432			min	-400.558	3	.791	15	-.081	1	0	3	0	1	-.006	4
433		8	max	273.157	2	2.591	4	-.003	15	0	1	0	15	-.002	15
434			min	-400.686	3	.61	15	-.081	1	0	3	0	1	-.008	4
435		9	max	272.987	2	1.821	4	-.003	15	0	1	0	15	-.002	15
436			min	-400.814	3	.429	15	-.081	1	0	3	0	1	-.009	4
437		10	max	272.816	2	1.051	4	-.003	15	0	1	0	15	-.002	15
438			min	-400.942	3	.248	15	-.081	1	0	3	0	1	-.009	4
439		11	max	272.646	2	.324	2	-.003	15	0	1	0	15	-.002	15
440			min	-401.069	3	.019	12	-.081	1	0	3	0	1	-.009	4
441		12	max	272.476	2	-.114	15	-.003	15	0	1	0	15	-.002	15
442			min	-401.197	3	-.489	4	-.081	1	0	3	0	1	-.009	4
443		13	max	272.305	2	-.295	15	-.003	15	0	1	0	15	-.002	15
444			min	-401.325	3	-1.259	4	-.081	1	0	3	0	1	-.009	4
445		14	max	272.135	2	-.476	15	-.003	15	0	1	0	15	-.002	15
446			min	-401.453	3	-2.029	4	-.081	1	0	3	0	1	-.008	4
447		15	max	271.965	2	-.657	15	-.003	15	0	1	0	15	-.002	15
448			min	-401.581	3	-2.799	4	-.081	1	0	3	0	1	-.007	4
449		16	max	271.794	2	-.838	15	-.003	15	0	1	0	15	-.001	15
450			min	-401.708	3	-3.569	4	-.081	1	0	3	0	1	-.006	4
451		17	max	271.624	2	-1.019	15	-.003	15	0	1	0	15	-.001	15
452			min	-401.836	3	-4.339	4	-.081	1	0	3	0	1	-.004	4
453		18	max	271.453	2	-1.2	15	-.003	15	0	1	0	15	0	15
454			min	-401.964	3	-5.109	4	-.081	1	0	3	0	1	-.002	4
455		19	max	271.283	2	-1.381	15	-.003	15	0	1	0	15	0	1
456			min	-402.092	3	-5.879	4	-.081	1	0	3	0	1	0	1
457	M12	1	max	1159.096	1	0	1	10.779	1	0	1	0	15	0	1
458			min	-151.133	3	0	1	.392	15	0	1	0	1	0	1
459		2	max	1159.267	1	0	1	10.779	1	0	1	0	1	0	1
460			min	-151.006	3	0	1	.392	15	0	1	0	12	0	1
461		3	max	1159.437	1	0	1	10.779	1	0	1	.002	1	0	1
462			min	-150.878	3	0	1	.392	15	0	1	0	15	0	1
463		4	max	1159.607	1	0	1	10.779	1	0	1	.003	1	0	1
464			min	-150.75	3	0	1	.392	15	0	1	0	15	0	1
465		5	max	1159.778	1	0	1	10.779	1	0	1	.004	1	0	1
466			min	-150.622	3	0	1	.392	15	0	1	0	15	0	1
467		6	max	1159.948	1	0	1	10.779	1	0	1	.006	1	0	1
468			min	-150.495	3	0	1	.392	15	0	1	0	15	0	1
469		7	max	1160.118	1	0	1	10.779	1	0	1	.007	1	0	1
470			min	-150.367	3	0	1	.392	15	0	1	0	15	0	1
471		8	max	1160.289	1	0	1	10.779	1	0	1	.008	1	0	1
472			min	-150.239	3	0	1	.392	15	0	1	0	15	0	1
473		9	max	1160.459	1	0	1	10.779	1	0	1	.009	1	0	1
474			min	-150.111	3	0	1	.392	15	0	1	0	15	0	1
475		10	max	1160.629	1	0	1	10.779	1	0	1	.011	1	0	1
476			min	-149.984	3	0	1	.392	15	0	1	0	15	0	1
477		11	max	1160.8	1	0	1	10.779	1	0	1	.012	1	0	1
478			min	-149.856	3	0	1	.392	15	0	1	0	15	0	1
479		12	max	1160.97	1	0	1	10.779	1	0	1	.013	1	0	1
480			min	-149.728	3	0	1	.392	15	0	1	0	15	0	1
481		13	max	1161.14	1	0	1	10.779	1	0	1	.014	1	0	1
482			min	-149.6	3	0	1	.392	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1161.311	1	0	1	10.779	1	0	1	.016	1	0	1
484		min	-149.473	3	0	1	.392	15	0	1	0	15	0	1
485	15	max	1161.481	1	0	1	10.779	1	0	1	.017	1	0	1
486		min	-149.345	3	0	1	.392	15	0	1	0	15	0	1
487	16	max	1161.652	1	0	1	10.779	1	0	1	.018	1	0	1
488		min	-149.217	3	0	1	.392	15	0	1	0	15	0	1
489	17	max	1161.822	1	0	1	10.779	1	0	1	.019	1	0	1
490		min	-149.089	3	0	1	.392	15	0	1	0	15	0	1
491	18	max	1161.992	1	0	1	10.779	1	0	1	.02	1	0	1
492		min	-148.962	3	0	1	.392	15	0	1	0	15	0	1
493	19	max	1162.163	1	0	1	10.779	1	0	1	.022	1	0	1
494		min	-148.834	3	0	1	.392	15	0	1	0	15	0	1
495	M1	1	max	169.946	1	458.559	3	-4.031	15	0	.263	1	0	3
496		min	6.185	15	-462.578	1	-110.332	1	0	3	.01	15	-.012	1
497	2	max	170.435	1	457.55	3	-4.031	15	0	1	.205	1	.232	1
498		min	6.332	15	-463.924	1	-110.332	1	0	3	.007	15	-.242	3
499	3	max	236.489	3	511.303	1	-3.994	15	0	3	.147	1	.466	1
500		min	-149.207	2	-325.323	3	-109.562	1	0	1	.005	15	-.473	3
501	4	max	236.857	3	509.957	1	-3.994	15	0	3	.089	1	.196	1
502		min	-148.717	2	-326.333	3	-109.562	1	0	1	.003	15	-.301	3
503	5	max	237.224	3	508.611	1	-3.994	15	0	3	.031	1	-.003	15
504		min	-148.227	2	-327.342	3	-109.562	1	0	1	.001	15	-.129	3
505	6	max	237.592	3	507.265	1	-3.994	15	0	3	0	15	.044	3
506		min	-147.737	2	-328.352	3	-109.562	1	0	1	-.026	1	-.34	1
507	7	max	237.959	3	505.919	1	-3.994	15	0	3	-.003	15	.218	3
508		min	-147.247	2	-329.361	3	-109.562	1	0	1	-.084	1	-.608	1
509	8	max	238.327	3	504.573	1	-3.994	15	0	3	-.005	15	.392	3
510		min	-146.757	2	-330.371	3	-109.562	1	0	1	-.142	1	-.874	1
511	9	max	248.769	3	30.705	2	-5.819	15	0	9	.083	1	.459	3
512		min	-77.396	2	.409	15	-159.457	1	0	3	.003	15	-.996	1
513	10	max	249.136	3	29.359	2	-5.819	15	0	9	0	15	.446	3
514		min	-76.906	2	.003	15	-159.457	1	0	3	-.001	1	-1.005	1
515	11	max	249.504	3	28.013	2	-5.819	15	0	9	-.003	15	.433	3
516		min	-76.416	2	-1.651	4	-159.457	1	0	3	-.085	1	-1.013	1
517	12	max	259.905	3	211.536	3	-3.896	15	0	1	.14	1	.377	3
518		min	-48.749	10	-536.872	1	-106.954	1	0	3	.005	15	-.894	1
519	13	max	260.272	3	210.527	3	-3.896	15	0	1	.084	1	.265	3
520		min	-48.34	10	-538.218	1	-106.954	1	0	3	.003	15	-.61	1
521	14	max	260.639	3	209.517	3	-3.896	15	0	1	.027	1	.155	3
522		min	-47.932	10	-539.564	1	-106.954	1	0	3	0	15	-.326	1
523	15	max	261.007	3	208.508	3	-3.896	15	0	1	-.001	15	.044	3
524		min	-47.524	10	-540.91	1	-106.954	1	0	3	-.029	1	-.041	1
525	16	max	261.374	3	207.498	3	-3.896	15	0	1	-.003	15	.245	1
526		min	-47.116	10	-542.256	1	-106.954	1	0	3	-.086	1	-.065	3
527	17	max	261.742	3	206.488	3	-3.896	15	0	1	-.005	15	.532	1
528		min	-46.707	10	-543.603	1	-106.954	1	0	3	-.142	1	-.175	3
529	18	max	-6.338	15	526.383	1	-4.285	15	0	3	-.007	15	.266	1
530		min	-170.626	1	-176.01	3	-117.339	1	0	1	-.203	1	-.087	3
531	19	max	-6.19	15	525.037	1	-4.285	15	0	3	-.01	15	.007	3
532		min	-170.136	1	-177.02	3	-117.339	1	0	1	-.265	1	-.011	1
533	M5	1	max	366.096	1	1528.11	3	0	1	0	0	1	.024	1
534		min	12.627	12	-1562.139	1	0	1	0	1	0	1	0	3
535	2	max	366.586	1	1527.1	3	0	1	0	1	0	1	.849	1
536		min	12.872	12	-1563.485	1	0	1	0	1	0	1	-.806	3
537	3	max	760.08	3	1572.462	1	0	1	0	1	0	1	1.636	1
538		min	-555.622	2	-1053.598	3	0	1	0	1	0	1	-1.581	3
539	4	max	760.448	3	1571.116	1	0	1	0	1	0	1	.807	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-555.132	2	-1054.607	3	0	1	0	1	0	1	-1.025	3
541		5	max	760.815	3	1569.77	1	0	1	0	1	0	1	.009	9
542			min	-554.642	2	-1055.617	3	0	1	0	1	0	1	-.468	3
543		6	max	761.182	3	1568.424	1	0	1	0	1	0	1	.089	3
544			min	-554.152	2	-1056.626	3	0	1	0	1	0	1	-.85	1
545		7	max	761.55	3	1567.078	1	0	1	0	1	0	1	.647	3
546			min	-553.662	2	-1057.636	3	0	1	0	1	0	1	-1.677	1
547		8	max	761.917	3	1565.732	1	0	1	0	1	0	1	1.205	3
548			min	-553.172	2	-1058.646	3	0	1	0	1	0	1	-2.503	1
549		9	max	780.624	3	101.311	2	0	1	0	1	0	1	1.39	3
550			min	-411.318	2	.407	15	0	1	0	1	0	1	-2.831	1
551		10	max	780.992	3	99.965	2	0	1	0	1	0	1	1.344	3
552			min	-410.828	2	.001	15	0	1	0	1	0	1	-2.859	1
553		11	max	781.359	3	98.619	2	0	1	0	1	0	1	1.299	3
554			min	-410.338	2	-1.497	4	0	1	0	1	0	1	-2.887	1
555		12	max	800.148	3	674.381	3	0	1	0	1	0	1	1.139	3
556			min	-268.491	2	-1675.084	1	0	1	0	1	0	1	-2.572	1
557		13	max	800.515	3	673.371	3	0	1	0	1	0	1	.783	3
558			min	-268.001	2	-1676.43	1	0	1	0	1	0	1	-1.688	1
559		14	max	800.883	3	672.362	3	0	1	0	1	0	1	.428	3
560			min	-267.511	2	-1677.776	1	0	1	0	1	0	1	-.803	1
561		15	max	801.25	3	671.352	3	0	1	0	1	0	1	.127	2
562			min	-267.021	2	-1679.122	1	0	1	0	1	0	1	-.004	13
563		16	max	801.618	3	670.343	3	0	1	0	1	0	1	.969	1
564			min	-266.531	2	-1680.468	1	0	1	0	1	0	1	-.28	3
565		17	max	801.985	3	669.333	3	0	1	0	1	0	1	1.856	1
566			min	-266.042	2	-1681.814	1	0	1	0	1	0	1	-.634	3
567		18	max	-13.037	12	1777.719	1	0	1	0	1	0	1	.96	1
568			min	-366.205	1	-602.415	3	0	1	0	1	0	1	-.331	3
569		19	max	-12.792	12	1776.373	1	0	1	0	1	0	1	.022	1
570			min	-365.715	1	-603.424	3	0	1	0	1	0	1	-.013	3
571	M9	1	max	169.946	1	458.559	3	110.332	1	0	3	-.01	15	0	3
572			min	6.185	15	-462.578	1	4.031	15	0	1	-.263	1	-.012	1
573		2	max	170.435	1	457.55	3	110.332	1	0	3	-.007	15	.232	1
574			min	6.332	15	-463.924	1	4.031	15	0	1	-.205	1	-.242	3
575		3	max	236.489	3	511.303	1	109.562	1	0	1	-.005	15	.466	1
576			min	-149.207	2	-325.323	3	3.994	15	0	3	-.147	1	-.473	3
577		4	max	236.857	3	509.957	1	109.562	1	0	1	-.003	15	.196	1
578			min	-148.717	2	-326.333	3	3.994	15	0	3	-.089	1	-.301	3
579		5	max	237.224	3	508.611	1	109.562	1	0	1	-.001	15	-.003	15
580			min	-148.227	2	-327.342	3	3.994	15	0	3	-.031	1	-.129	3
581		6	max	237.592	3	507.265	1	109.562	1	0	1	.026	1	.044	3
582			min	-147.737	2	-328.352	3	3.994	15	0	3	0	15	-.34	1
583		7	max	237.959	3	505.919	1	109.562	1	0	1	.084	1	.218	3
584			min	-147.247	2	-329.361	3	3.994	15	0	3	.003	15	-.608	1
585		8	max	238.327	3	504.573	1	109.562	1	0	1	.142	1	.392	3
586			min	-146.757	2	-330.371	3	3.994	15	0	3	.005	15	-.874	1
587		9	max	248.769	3	30.705	2	159.457	1	0	3	-.003	15	.459	3
588			min	-77.396	2	.409	15	5.819	15	0	9	-.083	1	-.996	1
589		10	max	249.136	3	29.359	2	159.457	1	0	3	.001	1	.446	3
590			min	-76.906	2	.003	15	5.819	15	0	9	0	15	-1.005	1
591		11	max	249.504	3	28.013	2	159.457	1	0	3	.085	1	.433	3
592			min	-76.416	2	-1.651	4	5.819	15	0	9	.003	15	-1.013	1
593		12	max	259.905	3	211.536	3	106.954	1	0	3	-.005	15	.377	3
594			min	-48.749	10	-536.872	1	3.896	15	0	1	-.14	1	-.894	1
595		13	max	260.272	3	210.527	3	106.954	1	0	3	-.003	15	.265	3
596			min	-48.34	10	-538.218	1	3.896	15	0	1	-.084	1	-.61	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	260.639	3	209.517	3	106.954	1	0	3	0	15	.155	3
598		min	-47.932	10	-539.564	1	3.896	15	0	1	-.027	1	-.326	1
599	15	max	261.007	3	208.508	3	106.954	1	0	3	.029	1	.044	3
600		min	-47.524	10	-540.91	1	3.896	15	0	1	.001	15	-.041	1
601	16	max	261.374	3	207.498	3	106.954	1	0	3	.086	1	.245	1
602		min	-47.116	10	-542.256	1	3.896	15	0	1	.003	15	-.065	3
603	17	max	261.742	3	206.488	3	106.954	1	0	3	.142	1	.532	1
604		min	-46.707	10	-543.603	1	3.896	15	0	1	.005	15	-.175	3
605	18	max	-6.338	15	526.383	1	117.339	1	0	1	.203	1	.266	1
606		min	-170.626	1	-176.01	3	4.285	15	0	3	.007	15	-.087	3
607	19	max	-6.19	15	525.037	1	117.339	1	0	1	.265	1	.007	3
608		min	-170.136	1	-177.02	3	4.285	15	0	3	.01	15	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.1	1	.004	3	7.968e-3	1	NC	1	NC	1
2			min	0	15	-.01	3	-.001	10	-8.014e-4	3	NC	1	NC	1
3		2	max	0	1	.241	3	.046	1	9.209e-3	1	NC	5	NC	2
4			min	0	15	-.134	1	.002	15	-8.193e-4	3	1003.94	3	5817.027	1
5		3	max	0	1	.445	3	.109	1	1.045e-2	1	NC	5	NC	3
6			min	0	15	-.319	1	.004	15	-8.372e-4	3	554.793	3	2364.41	1
7		4	max	0	1	.568	3	.164	1	1.169e-2	1	NC	5	NC	3
8			min	0	15	-.423	1	.006	15	-8.551e-4	3	436.341	3	1558.969	1
9		5	max	0	1	.596	3	.193	1	1.293e-2	1	NC	5	NC	3
10			min	0	15	-.433	1	.007	15	-8.73e-4	3	415.882	3	1324.131	1
11		6	max	0	1	.532	3	.187	1	1.417e-2	1	NC	5	NC	3
12			min	0	15	-.35	1	.007	15	-8.909e-4	3	465.299	3	1367.232	1
13		7	max	0	1	.394	3	.148	1	1.541e-2	1	NC	5	NC	3
14			min	0	15	-.195	1	.006	15	-9.088e-4	3	624.199	3	1734.265	1
15		8	max	0	1	.219	3	.087	1	1.665e-2	1	NC	4	NC	3
16			min	0	15	-.011	9	.003	15	-9.267e-4	3	1101.886	3	2968.84	1
17		9	max	0	1	.164	1	.027	1	1.789e-2	1	NC	4	NC	1
18			min	0	15	.005	15	-.003	10	-9.446e-4	3	3599.065	3	NC	1
19		10	max	0	1	.24	1	.014	3	1.913e-2	1	NC	3	NC	1
20			min	0	1	-.011	3	-.008	2	-9.625e-4	3	1795.246	1	NC	1
21		11	max	0	15	.164	1	.027	1	1.789e-2	1	NC	4	NC	1
22			min	0	1	.005	15	-.003	10	-9.446e-4	3	3599.065	3	NC	1
23		12	max	0	15	.219	3	.087	1	1.665e-2	1	NC	4	NC	3
24			min	0	1	-.011	9	.003	15	-9.267e-4	3	1101.886	3	2968.84	1
25		13	max	0	15	.394	3	.148	1	1.541e-2	1	NC	5	NC	3
26			min	0	1	-.195	1	.006	15	-9.088e-4	3	624.199	3	1734.265	1
27		14	max	0	15	.532	3	.187	1	1.417e-2	1	NC	5	NC	3
28			min	0	1	-.35	1	.007	15	-8.909e-4	3	465.299	3	1367.232	1
29		15	max	0	15	.596	3	.193	1	1.293e-2	1	NC	5	NC	3
30			min	0	1	-.433	1	.007	15	-8.73e-4	3	415.882	3	1324.131	1
31		16	max	0	15	.568	3	.164	1	1.169e-2	1	NC	5	NC	3
32			min	0	1	-.423	1	.006	15	-8.551e-4	3	436.341	3	1558.969	1
33		17	max	0	15	.445	3	.109	1	1.045e-2	1	NC	5	NC	3
34			min	0	1	-.319	1	.004	15	-8.372e-4	3	554.793	3	2364.41	1
35		18	max	0	15	.241	3	.046	1	9.209e-3	1	NC	5	NC	2
36			min	0	1	-.134	1	.002	15	-8.193e-4	3	1003.94	3	5817.027	1
37		19	max	0	15	.1	1	.004	3	7.968e-3	1	NC	1	NC	1
38			min	-.001	1	-.01	3	-.001	10	-8.014e-4	3	NC	1	NC	1
39	M14	1	max	0	1	.135	3	.004	3	5.022e-3	1	NC	1	NC	1
40			min	0	15	-.326	1	-.001	10	-2.449e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.369	3	.032	1	6.049e-3	1	NC	5	NC	2
42			min	0	15	-.68	1	.001	15	-2.995e-3	3	712.977	1	8458.12	1
43		3	max	0	1	.566	3	.088	1	7.077e-3	1	NC	15	NC	3
44			min	0	15	-.982	1	.003	15	-3.542e-3	3	384.342	1	2933.235	1
45		4	max	0	1	.7	3	.141	1	8.104e-3	1	NC	15	NC	3
46			min	0	15	-1.198	1	.005	15	-4.089e-3	3	289.172	1	1813.71	1
47		5	max	0	1	.759	3	.172	1	9.132e-3	1	9260.77	15	NC	3
48			min	0	15	-1.309	1	.006	15	-4.635e-3	3	256.332	1	1487.592	1
49		6	max	0	1	.744	3	.17	1	1.016e-2	1	9227.888	15	NC	3
50			min	0	15	-1.316	1	.006	15	-5.182e-3	3	254.451	1	1502.326	1
51		7	max	0	1	.667	3	.137	1	1.119e-2	1	NC	15	NC	3
52			min	0	15	-1.237	1	.005	15	-5.729e-3	3	276.764	1	1875.873	1
53		8	max	0	1	.556	3	.082	1	1.221e-2	1	NC	15	NC	3
54			min	0	15	-1.104	1	.003	15	-6.276e-3	3	323.782	1	3169.618	1
55		9	max	0	1	.45	3	.026	1	1.324e-2	1	NC	15	NC	1
56			min	0	15	-.972	1	-.002	10	-6.822e-3	3	390.133	1	NC	1
57		10	max	0	1	.4	3	.012	3	1.427e-2	1	NC	5	NC	1
58			min	0	1	-.909	1	-.008	2	-7.369e-3	3	432.146	1	NC	1
59		11	max	0	15	.45	3	.026	1	1.324e-2	1	NC	15	NC	1
60			min	0	1	-.972	1	-.002	10	-6.822e-3	3	390.133	1	NC	1
61		12	max	0	15	.556	3	.082	1	1.221e-2	1	NC	15	NC	3
62			min	0	1	-1.104	1	.003	15	-6.276e-3	3	323.782	1	3169.618	1
63		13	max	0	15	.667	3	.137	1	1.119e-2	1	NC	15	NC	3
64			min	0	1	-1.237	1	.005	15	-5.729e-3	3	276.764	1	1875.873	1
65		14	max	0	15	.744	3	.17	1	1.016e-2	1	9227.888	15	NC	3
66			min	0	1	-1.316	1	.006	15	-5.182e-3	3	254.451	1	1502.326	1
67		15	max	0	15	.759	3	.172	1	9.132e-3	1	9260.77	15	NC	3
68			min	0	1	-1.309	1	.006	15	-4.635e-3	3	256.332	1	1487.592	1
69		16	max	0	15	.7	3	.141	1	8.104e-3	1	NC	15	NC	3
70			min	0	1	-1.198	1	.005	15	-4.089e-3	3	289.172	1	1813.71	1
71		17	max	0	15	.566	3	.088	1	7.077e-3	1	NC	15	NC	3
72			min	0	1	-.982	1	.003	15	-3.542e-3	3	384.342	1	2933.235	1
73		18	max	0	15	.369	3	.032	1	6.049e-3	1	NC	5	NC	2
74			min	0	1	-.68	1	.001	15	-2.995e-3	3	712.977	1	8458.12	1
75		19	max	0	15	.135	3	.004	3	5.022e-3	1	NC	1	NC	1
76			min	0	1	-.326	1	-.001	10	-2.449e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.138	3	.004	3	2.054e-3	3	NC	1	NC	1
78			min	0	1	-.326	1	-.001	10	-5.122e-3	1	NC	1	NC	1
79		2	max	0	15	.281	3	.032	1	2.516e-3	3	NC	5	NC	2
80			min	0	1	-.712	1	.001	15	-6.176e-3	1	652.821	1	8419.386	1
81		3	max	0	15	.405	3	.089	1	2.978e-3	3	NC	15	NC	3
82			min	0	1	-1.04	1	.003	15	-7.23e-3	1	352.657	1	2925.416	1
83		4	max	0	15	.495	3	.142	1	3.44e-3	3	NC	15	NC	3
84			min	0	1	-1.272	1	.005	15	-8.284e-3	1	266.302	1	1809.995	1
85		5	max	0	15	.544	3	.172	1	3.901e-3	3	9270.963	15	NC	3
86			min	0	1	-1.387	1	.006	15	-9.338e-3	1	237.399	1	1484.862	1
87		6	max	0	15	.553	3	.17	1	4.363e-3	3	9240.041	15	NC	3
88			min	0	1	-1.386	1	.006	15	-1.039e-2	1	237.678	1	1499.502	1
89		7	max	0	15	.527	3	.137	1	4.825e-3	3	NC	15	NC	3
90			min	0	1	-1.288	1	.005	15	-1.145e-2	1	261.864	1	1871.607	1
91		8	max	0	15	.481	3	.082	1	5.287e-3	3	NC	15	NC	3
92			min	0	1	-1.133	1	.003	15	-1.25e-2	1	312.187	1	3158.103	1
93		9	max	0	15	.432	3	.026	1	5.749e-3	3	NC	15	NC	1
94			min	0	1	-.98	1	-.002	10	-1.355e-2	1	385.263	1	NC	1
95		10	max	0	1	.409	3	.011	3	6.211e-3	3	NC	5	NC	1
96			min	0	1	-.907	1	-.007	2	-1.461e-2	1	433.044	1	NC	1
97		11	max	0	1	.432	3	.026	1	5.749e-3	3	NC	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-.98	1	-.002	10	-1.355e-2	1	385.263	1	NC	1
99		max	0	1	.481	3	.082	1	5.287e-3	3	NC	15	NC	3
100		min	0	15	-1.133	1	.003	15	-1.25e-2	1	312.187	1	3158.103	1
101		max	0	1	.527	3	.137	1	4.825e-3	3	NC	15	NC	3
102		min	0	15	-1.288	1	.005	15	-1.145e-2	1	261.864	1	1871.607	1
103		max	0	1	.553	3	.17	1	4.363e-3	3	9240.041	15	NC	3
104		min	0	15	-1.386	1	.006	15	-1.039e-2	1	237.678	1	1499.502	1
105		max	0	1	.544	3	.172	1	3.901e-3	3	9270.963	15	NC	3
106		min	0	15	-1.387	1	.006	15	-9.338e-3	1	237.399	1	1484.862	1
107		max	0	1	.495	3	.142	1	3.44e-3	3	NC	15	NC	3
108		min	0	15	-1.272	1	.005	15	-8.284e-3	1	266.302	1	1809.995	1
109		max	0	1	.405	3	.089	1	2.978e-3	3	NC	15	NC	3
110		min	0	15	-1.04	1	.003	15	-7.23e-3	1	352.657	1	2925.416	1
111		max	0	1	.281	3	.032	1	2.516e-3	3	NC	5	NC	2
112		min	0	15	-.712	1	.001	15	-6.176e-3	1	652.821	1	8419.386	1
113		max	0	1	.138	3	.004	3	2.054e-3	3	NC	1	NC	1
114		min	0	15	-.326	1	-.001	10	-5.122e-3	1	NC	1	NC	1
115	M16	max	0	15	.098	1	.003	3	3.587e-3	3	NC	1	NC	1
116		min	-.001	1	-.045	3	-.001	10	-7.516e-3	1	NC	1	NC	1
117	2	max	0	15	.04	3	.045	1	4.257e-3	3	NC	5	NC	2
118		min	-.001	1	-.169	1	.002	15	-8.645e-3	1	944.728	1	5855.756	1
119	3	max	0	15	.107	3	.108	1	4.927e-3	3	NC	5	NC	3
120		min	0	1	-.381	1	.004	15	-9.774e-3	1	526.113	1	2372.253	1
121	4	max	0	15	.144	3	.163	1	5.596e-3	3	NC	5	NC	3
122		min	0	1	-.503	1	.006	15	-1.09e-2	1	419.793	1	1561.359	1
123	5	max	0	15	.144	3	.192	1	6.266e-3	3	NC	5	NC	3
124		min	0	1	-.516	1	.007	15	-1.203e-2	1	410.446	1	1324.282	1
125	6	max	0	15	.109	3	.187	1	6.936e-3	3	NC	5	NC	3
126		min	0	1	-.425	1	.007	15	-1.316e-2	1	481.832	1	1365.195	1
127	7	max	0	15	.047	3	.148	1	7.606e-3	3	NC	5	NC	3
128		min	0	1	-.252	1	.006	15	-1.429e-2	1	719.723	1	1727.22	1
129	8	max	0	15	.001	13	.088	1	8.275e-3	3	NC	3	NC	3
130		min	0	1	-.063	2	.003	15	-1.542e-2	1	1796.872	2	2938.324	1
131	9	max	0	15	.149	1	.028	1	8.945e-3	3	NC	4	NC	2
132		min	0	1	-.094	3	-.001	10	-1.655e-2	1	4862.527	1	9823.606	1
133	10	max	0	1	.235	1	.01	3	9.615e-3	3	NC	5	NC	1
134		min	0	1	-.123	3	-.007	2	-1.768e-2	1	1841.139	1	NC	1
135	11	max	0	1	.149	1	.028	1	8.945e-3	3	NC	4	NC	2
136		min	0	15	-.094	3	-.001	10	-1.655e-2	1	4862.527	1	9823.606	1
137	12	max	0	1	.001	13	.088	1	8.275e-3	3	NC	3	NC	3
138		min	0	15	-.063	2	.003	15	-1.542e-2	1	1796.872	2	2938.324	1
139	13	max	0	1	.047	3	.148	1	7.606e-3	3	NC	5	NC	3
140		min	0	15	-.252	1	.006	15	-1.429e-2	1	719.723	1	1727.22	1
141	14	max	0	1	.109	3	.187	1	6.936e-3	3	NC	5	NC	3
142		min	0	15	-.425	1	.007	15	-1.316e-2	1	481.832	1	1365.195	1
143	15	max	0	1	.144	3	.192	1	6.266e-3	3	NC	5	NC	3
144		min	0	15	-.516	1	.007	15	-1.203e-2	1	410.446	1	1324.282	1
145	16	max	0	1	.144	3	.163	1	5.596e-3	3	NC	5	NC	3
146		min	0	15	-.503	1	.006	15	-1.09e-2	1	419.793	1	1561.359	1
147	17	max	0	1	.107	3	.108	1	4.927e-3	3	NC	5	NC	3
148		min	0	15	-.381	1	.004	15	-9.774e-3	1	526.113	1	2372.253	1
149	18	max	.001	1	.04	3	.045	1	4.257e-3	3	NC	5	NC	2
150		min	0	15	-.169	1	.002	15	-8.645e-3	1	944.728	1	5855.756	1
151	19	max	.001	1	.098	1	.003	3	3.587e-3	3	NC	1	NC	1
152		min	0	15	-.045	3	-.001	10	-7.516e-3	1	NC	1	NC	1
153	M2	max	.005	1	.003	2	.008	1	-8.339e-6	15	NC	1	NC	2
154		min	-.004	3	-.006	3	0	15	-2.29e-4	1	NC	1	6513.796	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.005	1	.002	2	.008	1	-7.779e-6	15	NC	1	NC	2
156			min	-.004	3	-.006	3	0	15	-2.136e-4	1	NC	1	7105.572	1
157		3	max	.005	1	.002	2	.007	1	-7.219e-6	15	NC	1	NC	2
158			min	-.004	3	-.006	3	0	15	-1.982e-4	1	NC	1	7811.056	1
159		4	max	.005	1	.001	2	.006	1	-6.659e-6	15	NC	1	NC	2
160			min	-.004	3	-.005	3	0	15	-1.828e-4	1	NC	1	8660.462	1
161		5	max	.004	1	0	2	.006	1	-6.099e-6	15	NC	1	NC	2
162			min	-.003	3	-.005	3	0	15	-1.675e-4	1	NC	1	9695.053	1
163		6	max	.004	1	0	2	.005	1	-5.539e-6	15	NC	1	NC	1
164			min	-.003	3	-.005	3	0	15	-1.521e-4	1	NC	1	NC	1
165		7	max	.004	1	0	2	.004	1	-4.979e-6	15	NC	1	NC	1
166			min	-.003	3	-.005	3	0	15	-1.367e-4	1	NC	1	NC	1
167		8	max	.003	1	0	10	.004	1	-4.419e-6	15	NC	1	NC	1
168			min	-.003	3	-.005	3	0	15	-1.213e-4	1	NC	1	NC	1
169		9	max	.003	1	0	15	.003	1	-3.859e-6	15	NC	1	NC	1
170			min	-.002	3	-.004	3	0	15	-1.059e-4	1	NC	1	NC	1
171		10	max	.003	1	0	15	.003	1	-3.299e-6	15	NC	1	NC	1
172			min	-.002	3	-.004	3	0	15	-9.052e-5	1	NC	1	NC	1
173		11	max	.002	1	0	15	.002	1	-2.739e-6	15	NC	1	NC	1
174			min	-.002	3	-.004	3	0	15	-7.513e-5	1	NC	1	NC	1
175		12	max	.002	1	0	15	.002	1	-2.179e-6	15	NC	1	NC	1
176			min	-.002	3	-.004	3	0	15	-5.974e-5	1	NC	1	NC	1
177		13	max	.002	1	0	15	.001	1	-1.619e-6	15	NC	1	NC	1
178			min	-.001	3	-.003	3	0	15	-4.436e-5	1	NC	1	NC	1
179		14	max	.002	1	0	15	0	1	-1.059e-6	15	NC	1	NC	1
180			min	-.001	3	-.003	4	0	15	-2.897e-5	1	NC	1	NC	1
181		15	max	.001	1	0	15	0	1	-4.988e-7	15	NC	1	NC	1
182			min	0	3	-.003	4	0	15	-1.358e-5	1	NC	1	NC	1
183		16	max	0	1	0	15	0	1	1.808e-6	1	NC	1	NC	1
184			min	0	3	-.002	4	0	15	-1.892e-7	3	NC	1	NC	1
185		17	max	0	1	0	15	0	1	1.72e-5	1	NC	1	NC	1
186			min	0	3	-.001	4	0	15	5.506e-7	12	NC	1	NC	1
187		18	max	0	1	0	15	0	1	3.258e-5	1	NC	1	NC	1
188			min	0	3	0	4	0	15	1.181e-6	15	NC	1	NC	1
189		19	max	0	1	0	1	0	1	4.797e-5	1	NC	1	NC	1
190			min	0	1	0	1	0	1	1.741e-6	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-5.479e-7	15	NC	1	NC	1
192			min	0	1	0	1	0	1	-1.509e-5	1	NC	1	NC	1
193		2	max	0	3	0	15	0	1	1.079e-5	1	NC	1	NC	1
194			min	0	2	-.002	4	0	15	3.935e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	1	3.667e-5	1	NC	1	NC	1
196			min	0	2	-.003	4	0	15	1.335e-6	15	NC	1	NC	1
197		4	max	0	3	-.001	15	0	1	6.255e-5	1	NC	1	NC	1
198			min	0	2	-.005	4	0	15	2.276e-6	15	NC	1	NC	1
199		5	max	0	3	-.002	15	.001	1	8.843e-5	1	NC	1	NC	1
200			min	0	2	-.007	4	0	15	3.218e-6	15	NC	1	NC	1
201		6	max	0	3	-.002	15	.001	1	1.143e-4	1	NC	1	NC	1
202			min	0	2	-.009	4	0	15	4.159e-6	15	NC	1	NC	1
203		7	max	.001	3	-.002	15	.002	1	1.402e-4	1	NC	1	NC	1
204			min	0	2	-.01	4	0	15	5.1e-6	15	8923.064	4	NC	1
205		8	max	.001	3	-.003	15	.002	1	1.661e-4	1	NC	1	NC	1
206			min	0	2	-.012	4	0	15	6.042e-6	15	7983.869	4	NC	1
207		9	max	.002	3	-.003	15	.002	1	1.919e-4	1	NC	2	NC	1
208			min	-.001	2	-.013	4	0	15	6.983e-6	15	7425.501	4	NC	1
209		10	max	.002	3	-.003	15	.003	1	2.178e-4	1	NC	3	NC	1
210			min	-.001	2	-.013	4	0	15	7.924e-6	15	7149.826	4	NC	1
211		11	max	.002	3	-.003	15	.003	1	2.437e-4	1	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212			min	-.001	2	-.013	4	0	15	8.866e-6	15	7115.925	4	NC	1
213		12	max	.002	3	-.003	15	.004	1	2.696e-4	1	NC	2	NC	1
214			min	-.001	2	-.013	4	0	15	9.807e-6	15	7324.356	4	NC	1
215		13	max	.002	3	-.003	15	.004	1	2.955e-4	1	NC	1	NC	1
216			min	-.002	2	-.012	4	0	15	1.075e-5	15	7819.346	4	NC	1
217		14	max	.003	3	-.003	15	.005	1	3.213e-4	1	NC	1	NC	1
218			min	-.002	2	-.011	4	0	15	1.169e-5	15	8712.023	4	NC	1
219		15	max	.003	3	-.002	15	.005	1	3.472e-4	1	NC	1	NC	1
220			min	-.002	2	-.009	4	0	15	1.263e-5	15	NC	1	NC	1
221		16	max	.003	3	-.002	15	.006	1	3.731e-4	1	NC	1	NC	1
222			min	-.002	2	-.008	1	0	15	1.357e-5	15	NC	1	NC	1
223		17	max	.003	3	-.001	15	.007	1	3.99e-4	1	NC	1	NC	1
224			min	-.002	2	-.006	1	0	15	1.451e-5	15	NC	1	NC	1
225		18	max	.003	3	0	15	.007	1	4.249e-4	1	NC	1	NC	1
226			min	-.002	2	-.005	1	0	15	1.546e-5	15	NC	1	NC	1
227		19	max	.004	3	0	15	.008	1	4.507e-4	1	NC	1	NC	1
228			min	-.002	2	-.003	1	0	15	1.64e-5	15	NC	1	NC	1
229	M4	1	max	.003	1	.002	2	0	15	2.057e-5	1	NC	1	NC	3
230			min	0	3	-.004	3	-.008	1	7.573e-7	15	NC	1	3063.007	1
231		2	max	.003	1	.002	2	0	15	2.057e-5	1	NC	1	NC	3
232			min	0	3	-.003	3	-.007	1	7.573e-7	15	NC	1	3333.811	1
233		3	max	.002	1	.002	2	0	15	2.057e-5	1	NC	1	NC	3
234			min	0	3	-.003	3	-.007	1	7.573e-7	15	NC	1	3655.939	1
235		4	max	.002	1	.002	2	0	15	2.057e-5	1	NC	1	NC	2
236			min	0	3	-.003	3	-.006	1	7.573e-7	15	NC	1	4042.756	1
237		5	max	.002	1	.001	2	0	15	2.057e-5	1	NC	1	NC	2
238			min	0	3	-.003	3	-.005	1	7.573e-7	15	NC	1	4512.42	1
239		6	max	.002	1	.001	2	0	15	2.057e-5	1	NC	1	NC	2
240			min	0	3	-.003	3	-.005	1	7.573e-7	15	NC	1	5090.115	1
241		7	max	.002	1	.001	2	0	15	2.057e-5	1	NC	1	NC	2
242			min	0	3	-.002	3	-.004	1	7.573e-7	15	NC	1	5811.591	1
243		8	max	.002	1	.001	2	0	15	2.057e-5	1	NC	1	NC	2
244			min	0	3	-.002	3	-.004	1	7.573e-7	15	NC	1	6728.99	1
245		9	max	.002	1	.001	2	0	15	2.057e-5	1	NC	1	NC	2
246			min	0	3	-.002	3	-.003	1	7.573e-7	15	NC	1	7920.814	1
247		10	max	.001	1	0	2	0	15	2.057e-5	1	NC	1	NC	2
248			min	0	3	-.002	3	-.003	1	7.573e-7	15	NC	1	9509.795	1
249		11	max	.001	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
250			min	0	3	-.002	3	-.002	1	7.573e-7	15	NC	1	NC	1
251		12	max	.001	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
252			min	0	3	-.001	3	-.002	1	7.573e-7	15	NC	1	NC	1
253		13	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
254			min	0	3	-.001	3	-.001	1	7.573e-7	15	NC	1	NC	1
255		14	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
256			min	0	3	0	3	0	1	7.573e-7	15	NC	1	NC	1
257		15	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
258			min	0	3	0	3	0	1	7.573e-7	15	NC	1	NC	1
259		16	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
260			min	0	3	0	3	0	1	7.573e-7	15	NC	1	NC	1
261		17	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
262			min	0	3	0	3	0	1	7.573e-7	15	NC	1	NC	1
263		18	max	0	1	0	2	0	15	2.057e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	7.573e-7	15	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.057e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	7.573e-7	15	NC	1	NC	1
267	M6	1	max	.018	1	.013	2	0	1	0	1	NC	3	NC	1
268			min	-.014	3	-.018	3	0	1	0	1	4330.166	2	NC	1



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.017	1	.012	2	0	1	0	1	NC	3	NC	1
270		min	-.013	3	-.017	3	0	1	0	1	4780.697	2	NC	1
271	3	max	.016	1	.01	2	0	1	0	1	NC	1	NC	1
272		min	-.013	3	-.016	3	0	1	0	1	5330.92	2	NC	1
273	4	max	.015	1	.009	2	0	1	0	1	NC	1	NC	1
274		min	-.012	3	-.015	3	0	1	0	1	6011.74	2	NC	1
275	5	max	.014	1	.008	2	0	1	0	1	NC	1	NC	1
276		min	-.011	3	-.014	3	0	1	0	1	6867.498	2	NC	1
277	6	max	.013	1	.007	2	0	1	0	1	NC	1	NC	1
278		min	-.01	3	-.013	3	0	1	0	1	7963.709	2	NC	1
279	7	max	.012	1	.006	2	0	1	0	1	NC	1	NC	1
280		min	-.009	3	-.013	3	0	1	0	1	9400.641	2	NC	1
281	8	max	.011	1	.005	2	0	1	0	1	NC	1	NC	1
282		min	-.009	3	-.012	3	0	1	0	1	NC	1	NC	1
283	9	max	.01	1	.004	2	0	1	0	1	NC	1	NC	1
284		min	-.008	3	-.011	3	0	1	0	1	NC	1	NC	1
285	10	max	.009	1	.003	2	0	1	0	1	NC	1	NC	1
286		min	-.007	3	-.01	3	0	1	0	1	NC	1	NC	1
287	11	max	.008	1	.002	2	0	1	0	1	NC	1	NC	1
288		min	-.006	3	-.009	3	0	1	0	1	NC	1	NC	1
289	12	max	.007	1	.002	2	0	1	0	1	NC	1	NC	1
290		min	-.006	3	-.008	3	0	1	0	1	NC	1	NC	1
291	13	max	.006	1	.001	2	0	1	0	1	NC	1	NC	1
292		min	-.005	3	-.007	3	0	1	0	1	NC	1	NC	1
293	14	max	.005	1	0	2	0	1	0	1	NC	1	NC	1
294		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.003	3	-.004	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.002	3	-.002	3	0	1	0	1	NC	1	NC	1
301	18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
302		min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.002	3	0	1	0	1	NC	1	NC	1
309	3	max	.001	3	0	15	0	1	0	1	NC	1	NC	1
310		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
311	4	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
313	5	max	.002	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.002	2	-.007	4	0	1	0	1	NC	1	NC	1
315	6	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.003	2	-.009	4	0	1	0	1	NC	1	NC	1
317	7	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
318		min	-.003	2	-.01	4	0	1	0	1	9173.396	4	NC	1
319	8	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.004	2	-.012	4	0	1	0	1	8190.615	4	NC	1
321	9	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.005	2	-.013	4	0	1	0	1	7604.59	4	NC	1
323	10	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.005	2	-.013	4	0	1	0	1	7311.702	4	NC	1
325	11	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.006	2	-.013	4	0	1	0	1	7268.24	4	NC	1
327		12	max	.007	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.006	2	-.013	4	0	1	0	1	7473.548	4	NC	1
329		13	max	.007	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.007	2	-.012	4	0	1	0	1	7971.854	4	NC	1
331		14	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.008	2	-.011	4	0	1	0	1	8875.697	4	NC	1
333		15	max	.009	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.008	2	-.011	1	0	1	0	1	NC	1	NC	1
335		16	max	.009	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.009	2	-.01	1	0	1	0	1	NC	1	NC	1
337		17	max	.01	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.009	2	-.009	1	0	1	0	1	NC	1	NC	1
339		18	max	.01	3	0	15	0	1	0	1	NC	1	NC	1
340			min	-.01	2	-.008	1	0	1	0	1	NC	1	NC	1
341		19	max	.011	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.01	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.009	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.011	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.009	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.01	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	1	.008	2	0	1	0	1	NC	1	NC	1
348			min	-.001	3	-.01	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.008	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.009	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.007	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.009	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	1	.007	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.006	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.006	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.005	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.005	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.004	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.004	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.002	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.005	1	.003	2	0	15	2.29e-4	1	NC	1	NC	2
382			min	-.004	3	-.006	3	-.008	1	8.339e-6	15	NC	1	6513.796	1



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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.005	1	.002	2	0	15	2.136e-4	1	NC	1	NC	2
384		min	-.004	3	-.006	3	-.008	1	7.779e-6	15	NC	1	7105.572	1
385	3	max	.005	1	.002	2	0	15	1.982e-4	1	NC	1	NC	2
386		min	-.004	3	-.006	3	-.007	1	7.219e-6	15	NC	1	7811.056	1
387	4	max	.005	1	.001	2	0	15	1.828e-4	1	NC	1	NC	2
388		min	-.004	3	-.005	3	-.006	1	6.659e-6	15	NC	1	8660.462	1
389	5	max	.004	1	0	2	0	15	1.675e-4	1	NC	1	NC	2
390		min	-.003	3	-.005	3	-.006	1	6.099e-6	15	NC	1	9695.053	1
391	6	max	.004	1	0	2	0	15	1.521e-4	1	NC	1	NC	1
392		min	-.003	3	-.005	3	-.005	1	5.539e-6	15	NC	1	NC	1
393	7	max	.004	1	0	2	0	15	1.367e-4	1	NC	1	NC	1
394		min	-.003	3	-.005	3	-.004	1	4.979e-6	15	NC	1	NC	1
395	8	max	.003	1	0	10	0	15	1.213e-4	1	NC	1	NC	1
396		min	-.003	3	-.005	3	-.004	1	4.419e-6	15	NC	1	NC	1
397	9	max	.003	1	0	15	0	15	1.059e-4	1	NC	1	NC	1
398		min	-.002	3	-.004	3	-.003	1	3.859e-6	15	NC	1	NC	1
399	10	max	.003	1	0	15	0	15	9.052e-5	1	NC	1	NC	1
400		min	-.002	3	-.004	3	-.003	1	3.299e-6	15	NC	1	NC	1
401	11	max	.002	1	0	15	0	15	7.513e-5	1	NC	1	NC	1
402		min	-.002	3	-.004	3	-.002	1	2.739e-6	15	NC	1	NC	1
403	12	max	.002	1	0	15	0	15	5.974e-5	1	NC	1	NC	1
404		min	-.002	3	-.004	3	-.002	1	2.179e-6	15	NC	1	NC	1
405	13	max	.002	1	0	15	0	15	4.436e-5	1	NC	1	NC	1
406		min	-.001	3	-.003	3	-.001	1	1.619e-6	15	NC	1	NC	1
407	14	max	.002	1	0	15	0	15	2.897e-5	1	NC	1	NC	1
408		min	-.001	3	-.003	4	0	1	1.059e-6	15	NC	1	NC	1
409	15	max	.001	1	0	15	0	15	1.358e-5	1	NC	1	NC	1
410		min	0	3	-.003	4	0	1	4.988e-7	15	NC	1	NC	1
411	16	max	0	1	0	15	0	15	1.892e-7	3	NC	1	NC	1
412		min	0	3	-.002	4	0	1	-1.808e-6	1	NC	1	NC	1
413	17	max	0	1	0	15	0	15	-5.506e-7	12	NC	1	NC	1
414		min	0	3	-.001	4	0	1	-1.72e-5	1	NC	1	NC	1
415	18	max	0	1	0	15	0	15	-1.181e-6	15	NC	1	NC	1
416		min	0	3	0	4	0	1	-3.258e-5	1	NC	1	NC	1
417	19	max	0	1	0	1	0	1	-1.741e-6	15	NC	1	NC	1
418		min	0	1	0	1	0	1	-4.797e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	1.509e-5	1	NC	1	NC	1
420		min	0	1	0	1	0	1	5.479e-7	15	NC	1	NC	1
421	2	max	0	3	0	15	0	15	-3.935e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	1	-1.079e-5	1	NC	1	NC	1
423	3	max	0	3	0	15	0	15	-1.335e-6	15	NC	1	NC	1
424		min	0	2	-.003	4	0	1	-3.667e-5	1	NC	1	NC	1
425	4	max	0	3	-.001	15	0	15	-2.276e-6	15	NC	1	NC	1
426		min	0	2	-.005	4	0	1	-6.255e-5	1	NC	1	NC	1
427	5	max	0	3	-.002	15	0	15	-3.218e-6	15	NC	1	NC	1
428		min	0	2	-.007	4	-.001	1	-8.843e-5	1	NC	1	NC	1
429	6	max	0	3	-.002	15	0	15	-4.159e-6	15	NC	1	NC	1
430		min	0	2	-.009	4	-.001	1	-1.143e-4	1	NC	1	NC	1
431	7	max	.001	3	-.002	15	0	15	-5.1e-6	15	NC	1	NC	1
432		min	0	2	-.01	4	-.002	1	-1.402e-4	1	8923.064	4	NC	1
433	8	max	.001	3	-.003	15	0	15	-6.042e-6	15	NC	1	NC	1
434		min	0	2	-.012	4	-.002	1	-1.661e-4	1	7983.869	4	NC	1
435	9	max	.002	3	-.003	15	0	15	-6.983e-6	15	NC	2	NC	1
436		min	-.001	2	-.013	4	-.002	1	-1.919e-4	1	7425.501	4	NC	1
437	10	max	.002	3	-.003	15	0	15	-7.924e-6	15	NC	3	NC	1
438		min	-.001	2	-.013	4	-.003	1	-2.178e-4	1	7149.826	4	NC	1
439	11	max	.002	3	-.003	15	0	15	-8.866e-6	15	NC	3	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.001	2	-.013	4	-.003	1	-2.437e-4	1	7115.925	4	NC	1
441		max	.002	3	-.003	15	0	15	-9.807e-6	15	NC	2	NC	1
442		min	-.001	2	-.013	4	-.004	1	-2.696e-4	1	7324.356	4	NC	1
443		max	.002	3	-.003	15	0	15	-1.075e-5	15	NC	1	NC	1
444		min	-.002	2	-.012	4	-.004	1	-2.955e-4	1	7819.346	4	NC	1
445		max	.003	3	-.003	15	0	15	-1.169e-5	15	NC	1	NC	1
446		min	-.002	2	-.011	4	-.005	1	-3.213e-4	1	8712.023	4	NC	1
447		max	.003	3	-.002	15	0	15	-1.263e-5	15	NC	1	NC	1
448		min	-.002	2	-.009	4	-.005	1	-3.472e-4	1	NC	1	NC	1
449		max	.003	3	-.002	15	0	15	-1.357e-5	15	NC	1	NC	1
450		min	-.002	2	-.008	1	-.006	1	-3.731e-4	1	NC	1	NC	1
451		max	.003	3	-.001	15	0	15	-1.451e-5	15	NC	1	NC	1
452		min	-.002	2	-.006	1	-.007	1	-3.99e-4	1	NC	1	NC	1
453		max	.003	3	0	15	0	15	-1.546e-5	15	NC	1	NC	1
454		min	-.002	2	-.005	1	-.007	1	-4.249e-4	1	NC	1	NC	1
455		max	.004	3	0	15	0	15	-1.64e-5	15	NC	1	NC	1
456		min	-.002	2	-.003	1	-.008	1	-4.507e-4	1	NC	1	NC	1
457	M12	max	.003	1	.002	2	.008	1	-7.573e-7	15	NC	1	NC	3
458		min	0	3	-.004	3	0	15	-2.057e-5	1	NC	1	3063.007	1
459		max	.003	1	.002	2	.007	1	-7.573e-7	15	NC	1	NC	3
460		min	0	3	-.003	3	0	15	-2.057e-5	1	NC	1	3333.811	1
461		max	.002	1	.002	2	.007	1	-7.573e-7	15	NC	1	NC	3
462		min	0	3	-.003	3	0	15	-2.057e-5	1	NC	1	3655.939	1
463		max	.002	1	.002	2	.006	1	-7.573e-7	15	NC	1	NC	2
464		min	0	3	-.003	3	0	15	-2.057e-5	1	NC	1	4042.756	1
465		max	.002	1	.001	2	.005	1	-7.573e-7	15	NC	1	NC	2
466		min	0	3	-.003	3	0	15	-2.057e-5	1	NC	1	4512.42	1
467		max	.002	1	.001	2	.005	1	-7.573e-7	15	NC	1	NC	2
468		min	0	3	-.003	3	0	15	-2.057e-5	1	NC	1	5090.115	1
469		max	.002	1	.001	2	.004	1	-7.573e-7	15	NC	1	NC	2
470		min	0	3	-.002	3	0	15	-2.057e-5	1	NC	1	5811.591	1
471		max	.002	1	.001	2	.004	1	-7.573e-7	15	NC	1	NC	2
472		min	0	3	-.002	3	0	15	-2.057e-5	1	NC	1	6728.99	1
473		max	.002	1	.001	2	.003	1	-7.573e-7	15	NC	1	NC	2
474		min	0	3	-.002	3	0	15	-2.057e-5	1	NC	1	7920.814	1
475		max	.001	1	0	2	.003	1	-7.573e-7	15	NC	1	NC	2
476		min	0	3	-.002	3	0	15	-2.057e-5	1	NC	1	9509.795	1
477		max	.001	1	0	2	.002	1	-7.573e-7	15	NC	1	NC	1
478		min	0	3	-.002	3	0	15	-2.057e-5	1	NC	1	NC	1
479		max	.001	1	0	2	.002	1	-7.573e-7	15	NC	1	NC	1
480		min	0	3	-.001	3	0	15	-2.057e-5	1	NC	1	NC	1
481		max	0	1	0	2	.001	1	-7.573e-7	15	NC	1	NC	1
482		min	0	3	-.001	3	0	15	-2.057e-5	1	NC	1	NC	1
483		max	0	1	0	2	0	1	-7.573e-7	15	NC	1	NC	1
484		min	0	3	0	3	0	15	-2.057e-5	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-7.573e-7	15	NC	1	NC	1
486		min	0	3	0	3	0	15	-2.057e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-7.573e-7	15	NC	1	NC	1
488		min	0	3	0	3	0	15	-2.057e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-7.573e-7	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-2.057e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-7.573e-7	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-2.057e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-7.573e-7	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-2.057e-5	1	NC	1	NC	1
495	M1	max	.004	3	.1	1	.001	1	1.735e-2	1	NC	1	NC	1
496		min	-.001	10	-.01	3	0	15	-1.86e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.004	3	.049	1	0	15	8.43e-3	1	NC	3	NC	1
498			min	-.001	2	-.004	3	-.006	1	-9.202e-3	3	2251.688	1	NC	1
499		3	max	.004	3	.006	3	0	15	1.351e-5	10	NC	5	NC	1
500			min	-.001	2	-.006	1	-.009	1	-1.742e-4	1	1076.247	1	NC	1
501		4	max	.004	3	.025	3	0	15	4.665e-3	1	NC	5	NC	1
502			min	-.001	2	-.07	1	-.008	1	-3.268e-3	3	671.539	1	NC	1
503		5	max	.004	3	.05	3	0	15	9.505e-3	1	NC	15	NC	1
504			min	-.001	10	-.138	1	-.006	1	-6.445e-3	3	479.957	1	NC	1
505		6	max	.004	3	.078	3	0	15	1.434e-2	1	NC	15	NC	1
506			min	-.001	10	-.205	1	-.002	1	-9.622e-3	3	375.201	1	NC	1
507		7	max	.004	3	.105	3	0	1	1.918e-2	1	9674.843	15	NC	1
508			min	-.001	10	-.264	1	0	12	-1.28e-2	3	313.745	1	NC	1
509		8	max	.004	3	.128	3	0	1	2.402e-2	1	8592.37	15	NC	1
510			min	-.001	10	-.312	1	0	15	-1.598e-2	3	277.557	1	NC	1
511		9	max	.004	3	.142	3	0	15	2.641e-2	1	8028.295	15	NC	1
512			min	-.001	10	-.342	1	0	1	-1.598e-2	3	258.779	1	NC	1
513		10	max	.004	3	.148	3	0	1	2.716e-2	1	7856.573	15	NC	1
514			min	-.001	10	-.352	1	0	12	-1.388e-2	3	253.149	1	NC	1
515		11	max	.004	3	.144	3	0	1	2.791e-2	1	8028.101	15	NC	1
516			min	-.001	10	-.341	1	0	15	-1.178e-2	3	259.059	1	NC	1
517		12	max	.004	3	.132	3	0	15	2.631e-2	1	8591.965	15	NC	1
518			min	-.001	10	-.311	1	-.001	1	-9.738e-3	3	278.437	1	NC	1
519		13	max	.004	3	.112	3	0	15	2.117e-2	1	9674.131	15	NC	1
520			min	-.001	10	-.262	1	0	1	-7.794e-3	3	315.932	1	NC	1
521		14	max	.003	3	.087	3	.002	1	1.602e-2	1	NC	15	NC	1
522			min	-.001	10	-.202	1	0	15	-5.85e-3	3	379.928	1	NC	1
523		15	max	.003	3	.059	3	.005	1	1.087e-2	1	NC	15	NC	1
524			min	-.001	10	-.135	1	0	15	-3.905e-3	3	489.752	1	NC	1
525		16	max	.003	3	.03	3	.008	1	5.719e-3	1	NC	5	NC	1
526			min	-.001	10	-.067	1	0	15	-1.961e-3	3	692.281	1	NC	1
527		17	max	.003	3	.002	3	.008	1	5.702e-4	1	NC	5	NC	1
528			min	-.001	10	-.004	2	0	15	-1.654e-5	3	1123.4	1	NC	1
529		18	max	.003	3	.05	1	.006	1	1.002e-2	1	NC	4	NC	1
530			min	-.001	10	-.022	3	0	15	-3.131e-3	3	2371.815	1	NC	1
531		19	max	.003	3	.098	1	0	15	1.982e-2	1	NC	1	NC	1
532			min	-.001	10	-.045	3	-.001	1	-6.363e-3	3	NC	1	NC	1
533	M5	1	max	.014	3	.24	1	0	1	0	1	NC	1	NC	1
534			min	-.008	2	-.011	3	0	1	0	1	NC	1	NC	1
535		2	max	.014	3	.117	1	0	1	0	1	NC	5	NC	1
536			min	-.008	2	-.004	3	0	1	0	1	930.897	1	NC	1
537		3	max	.014	3	.02	3	0	1	0	1	NC	15	NC	1
538			min	-.009	2	-.022	1	0	1	0	1	435.183	1	NC	1
539		4	max	.013	3	.073	3	0	1	0	1	9121.384	15	NC	1
540			min	-.008	2	-.192	1	0	1	0	1	264.074	1	NC	1
541		5	max	.013	3	.146	3	0	1	0	1	6386.094	15	NC	1
542			min	-.008	2	-.379	1	0	1	0	1	184.594	1	NC	1
543		6	max	.013	3	.229	3	0	1	0	1	4918.287	15	NC	1
544			min	-.008	2	-.564	1	0	1	0	1	141.966	1	NC	1
545		7	max	.013	3	.311	3	0	1	0	1	4070.299	15	NC	1
546			min	-.008	2	-.733	1	0	1	0	1	117.347	1	NC	1
547		8	max	.012	3	.379	3	0	1	0	1	3577.092	15	NC	1
548			min	-.008	2	-.868	1	0	1	0	1	103.033	1	NC	1
549		9	max	.012	3	.423	3	0	1	0	1	3324.107	15	NC	1
550			min	-.007	2	-.953	1	0	1	0	1	95.697	1	NC	1
551		10	max	.012	3	.439	3	0	1	0	1	3247.877	15	NC	1
552			min	-.007	2	-.982	1	0	1	0	1	93.51	1	NC	1
553		11	max	.011	3	.428	3	0	1	0	1	3324.174	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.007	2	-.953	1	0	1	0	1	95.81	1	NC	1
555		12	max	.011	3	.391	3	0	1	0	1	3577.253	15	NC	1
556			min	-.007	2	-.866	1	0	1	0	1	103.408	1	NC	1
557		13	max	.011	3	.331	3	0	1	0	1	4070.634	15	NC	1
558			min	-.007	2	-.727	1	0	1	0	1	118.322	1	NC	1
559		14	max	.011	3	.256	3	0	1	0	1	4918.953	15	NC	1
560			min	-.007	2	-.555	1	0	1	0	1	144.16	1	NC	1
561		15	max	.01	3	.172	3	0	1	0	1	6387.424	15	NC	1
562			min	-.007	2	-.366	1	0	1	0	1	189.345	1	NC	1
563		16	max	.01	3	.087	3	0	1	0	1	9124.188	15	NC	1
564			min	-.007	2	-.179	1	0	1	0	1	274.69	1	NC	1
565		17	max	.01	3	.007	3	0	1	0	1	NC	15	NC	1
566			min	-.007	2	-.012	1	0	1	0	1	460.916	1	NC	1
567		18	max	.01	3	.121	1	0	1	0	1	NC	5	NC	1
568			min	-.007	2	-.061	3	0	1	0	1	999.663	1	NC	1
569		19	max	.01	3	.235	1	0	1	0	1	NC	1	NC	1
570			min	-.007	2	-.123	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.004	3	.1	1	0	15	1.86e-2	3	NC	1	NC	1
572			min	-.001	10	-.01	3	-.001	1	-1.735e-2	1	NC	1	NC	1
573		2	max	.004	3	.049	1	.006	1	9.202e-3	3	NC	3	NC	1
574			min	-.001	2	-.004	3	0	15	-8.43e-3	1	2251.688	1	NC	1
575		3	max	.004	3	.006	3	.009	1	1.742e-4	1	NC	5	NC	1
576			min	-.001	2	-.006	1	0	15	-1.351e-5	10	1076.247	1	NC	1
577		4	max	.004	3	.025	3	.008	1	3.268e-3	3	NC	5	NC	1
578			min	-.001	2	-.07	1	0	15	-4.665e-3	1	671.539	1	NC	1
579		5	max	.004	3	.05	3	.006	1	6.445e-3	3	NC	15	NC	1
580			min	-.001	10	-.138	1	0	15	-9.505e-3	1	479.957	1	NC	1
581		6	max	.004	3	.078	3	.002	1	9.622e-3	3	NC	15	NC	1
582			min	-.001	10	-.205	1	0	15	-1.434e-2	1	375.201	1	NC	1
583		7	max	.004	3	.105	3	0	12	1.28e-2	3	9674.843	15	NC	1
584			min	-.001	10	-.264	1	0	1	-1.918e-2	1	313.745	1	NC	1
585		8	max	.004	3	.128	3	0	15	1.598e-2	3	8592.37	15	NC	1
586			min	-.001	10	-.312	1	0	1	-2.402e-2	1	277.557	1	NC	1
587		9	max	.004	3	.142	3	0	1	1.598e-2	3	8028.295	15	NC	1
588			min	-.001	10	-.342	1	0	15	-2.641e-2	1	258.779	1	NC	1
589		10	max	.004	3	.148	3	0	12	1.388e-2	3	7856.573	15	NC	1
590			min	-.001	10	-.352	1	0	1	-2.716e-2	1	253.149	1	NC	1
591		11	max	.004	3	.144	3	0	15	1.178e-2	3	8028.101	15	NC	1
592			min	-.001	10	-.341	1	0	1	-2.791e-2	1	259.059	1	NC	1
593		12	max	.004	3	.132	3	.001	1	9.738e-3	3	8591.965	15	NC	1
594			min	-.001	10	-.311	1	0	15	-2.631e-2	1	278.437	1	NC	1
595		13	max	.004	3	.112	3	0	1	7.794e-3	3	9674.131	15	NC	1
596			min	-.001	10	-.262	1	0	15	-2.117e-2	1	315.932	1	NC	1
597		14	max	.003	3	.087	3	0	15	5.85e-3	3	NC	15	NC	1
598			min	-.001	10	-.202	1	-.002	1	-1.602e-2	1	379.928	1	NC	1
599		15	max	.003	3	.059	3	0	15	3.905e-3	3	NC	15	NC	1
600			min	-.001	10	-.135	1	-.005	1	-1.087e-2	1	489.752	1	NC	1
601		16	max	.003	3	.03	3	0	15	1.961e-3	3	NC	5	NC	1
602			min	-.001	10	-.067	1	-.008	1	-5.719e-3	1	692.281	1	NC	1
603		17	max	.003	3	.002	3	0	15	1.654e-5	3	NC	5	NC	1
604			min	-.001	10	-.004	2	-.008	1	-5.702e-4	1	1123.4	1	NC	1
605		18	max	.003	3	.05	1	0	15	3.131e-3	3	NC	4	NC	1
606			min	-.001	10	-.022	3	-.006	1	-1.002e-2	1	2371.815	1	NC	1
607		19	max	.003	3	.098	1	.001	1	6.363e-3	3	NC	1	NC	1
608			min	-.001	10	-.045	3	0	15	-1.982e-2	1	NC	1	NC	1



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

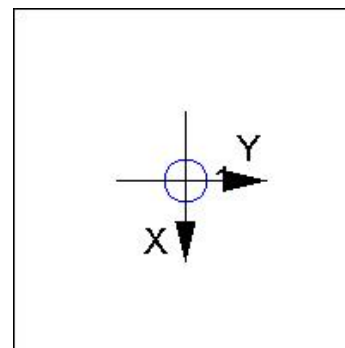
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1723  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Engineer:	HCV	Page:	5/5
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Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

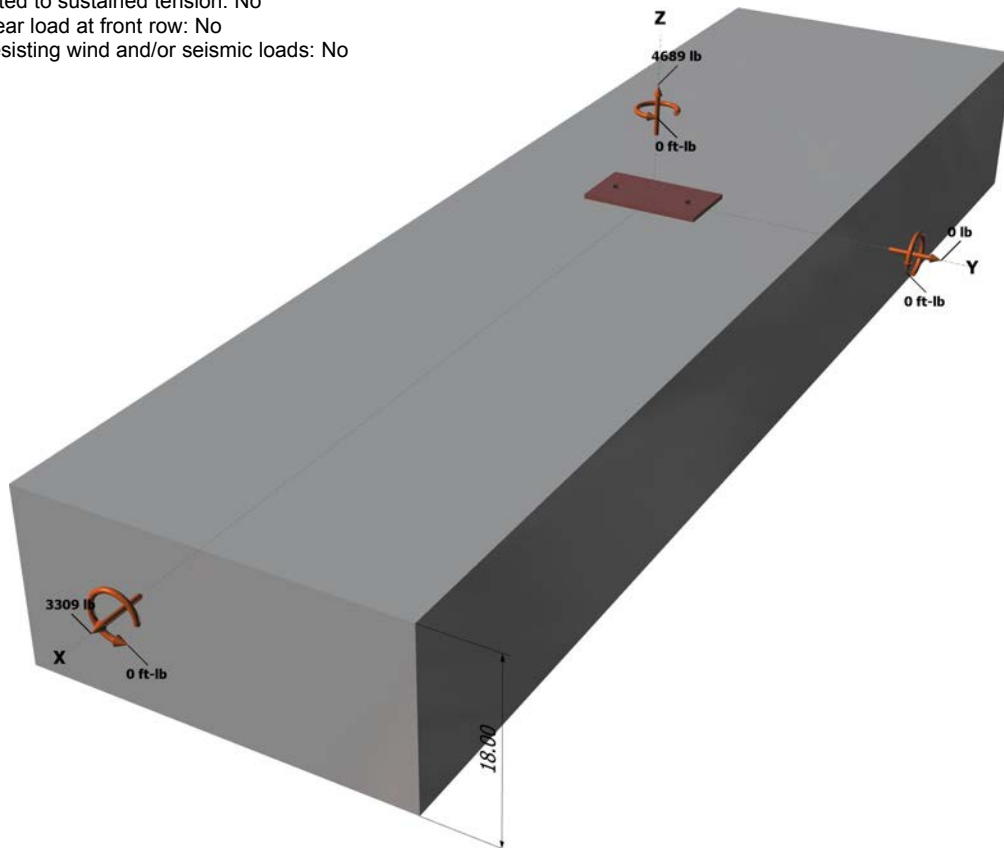
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

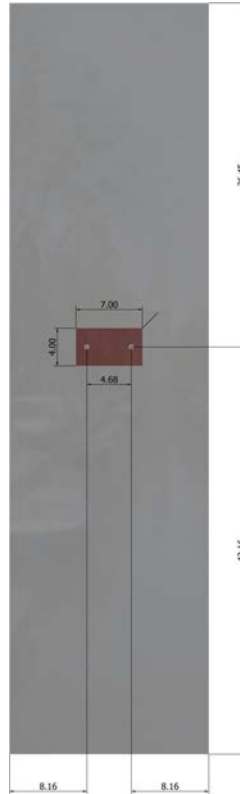
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.