

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

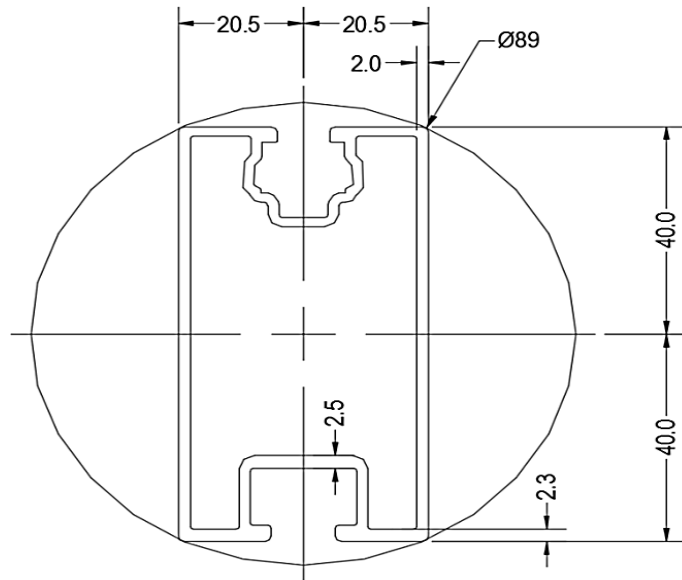
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	99 in
$\Phi F_{ty}$ STRONG-AXIS =	28.68 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	1.289 k-ft
$M_z$ =	0.183 k-ft
$M_{y \text{ allowable}}$ =	1.782 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>94%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.66 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.647 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.203 k
$M_{y \text{ allowable}}$ =	1.455 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>46%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.057 k-ft
$P_n$ =	0.327 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>16%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.164 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>4%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.408 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<u>17%</u>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.007 k-ft
$P_n$ =	0.264 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>18%</u>



A cross brace kit is required every 11 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

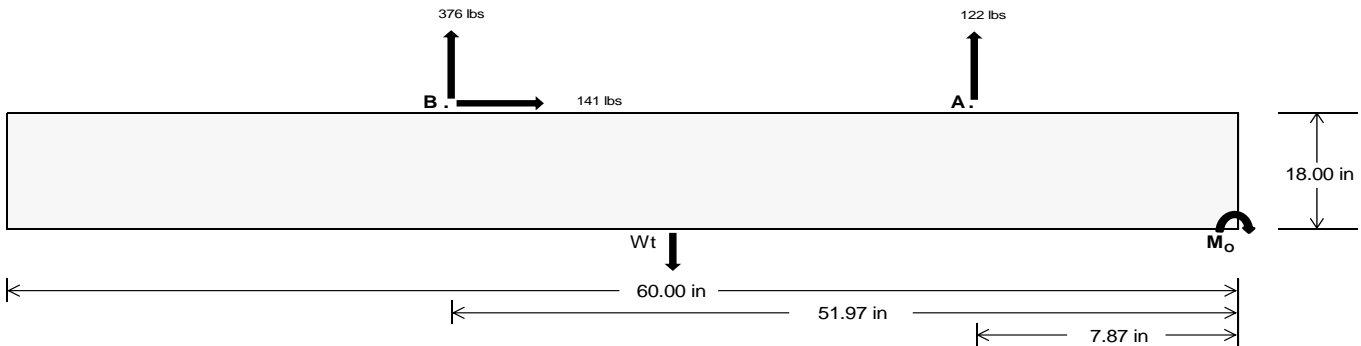
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>511.37</u>	<u>1566.57</u>	k
Compressive Load =	<u>2277.82</u>	<u>1670.25</u>	k
Lateral Load =	<u>46.05</u>	<u>588.58</u>	k
Moment (Weak Axis) =	<u>0.07</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 23021.8$  in-lbs  
Resisting Force Required = 767.39 lbs  
S.F. = 1.67  
Weight Required = 1278.99 lbs  
Minimum Width = 23 in  
Weight Provided = 2084.38 lbs

### Sliding

Force = 141.44 lbs  
Friction = 0.4  
Weight Required = 353.59 lbs  
Resisting Weight = 2084.38 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 141.44 lbs  
Cohesion = 130 psf  
Area = 9.58 ft<sup>2</sup>  
Resisting = 1042.19 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 23in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 23in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 23in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.92 \text{ ft}) =$

Ballast Width			
23 in	24 in	25 in	26 in
2084 lbs	2175 lbs	2266 lbs	2356 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in
$F_A$	860 lbs	860 lbs	860 lbs	860 lbs	648 lbs	648 lbs	648 lbs	648 lbs	1072 lbs	1072 lbs	1072 lbs	1072 lbs	-243 lbs	-243 lbs	-243 lbs	-243 lbs
$F_B$	632 lbs	632 lbs	632 lbs	632 lbs	474 lbs	474 lbs	474 lbs	474 lbs	785 lbs	785 lbs	785 lbs	785 lbs	-751 lbs	-751 lbs	-751 lbs	-751 lbs
$F_V$	60 lbs	60 lbs	60 lbs	60 lbs	252 lbs	252 lbs	252 lbs	252 lbs	231 lbs	231 lbs	231 lbs	231 lbs	-283 lbs	-283 lbs	-283 lbs	-283 lbs
$P_{total}$	3576 lbs	3667 lbs	3758 lbs	3848 lbs	3206 lbs	3297 lbs	3387 lbs	3478 lbs	3942 lbs	4032 lbs	4123 lbs	4213 lbs	256 lbs	311 lbs	365 lbs	419 lbs
$M$	517 lbs-ft	517 lbs-ft	517 lbs-ft	517 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft	885 lbs-ft	885 lbs-ft	885 lbs-ft	885 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft
$e$	0.14 ft	0.14 ft	0.14 ft	0.13 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.22 ft	0.22 ft	0.21 ft	0.21 ft	1.96 ft	1.62 ft	1.38 ft	1.20 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	308.4 psf	304.7 psf	301.2 psf	298.0 psf	246.1 psf	244.9 psf	243.8 psf	242.8 psf	300.4 psf	297.0 psf	293.8 psf	290.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	437.9 psf	428.7 psf	420.3 psf	412.5 psf	423.0 psf	414.4 psf	406.6 psf	399.3 psf	522.1 psf	509.4 psf	497.8 psf	487.0 psf	165.4 psf	117.4 psf	104.0 psf	99.2 psf

Maximum Bearing Pressure = 522 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

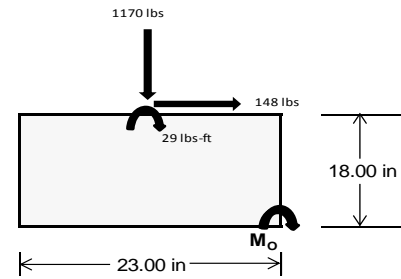
### Overturning Check

$M_o = 870.8 \text{ ft-lbs}$   
 Resisting Force Required = 908.62 lbs  
 S.F. = 1.67  
 Weight Required = 1514.36 lbs  
 Minimum Width = **23 in**  
 Weight Provided = 2084.38 lbs

*A minimum 60in long x 23in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	23 in			23 in			23 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	137 lbs	206 lbs	88 lbs	423 lbs	1170 lbs	384 lbs	73 lbs	25 lbs	28 lbs
$F_v$	24 lbs	195 lbs	25 lbs	16 lbs	148 lbs	19 lbs	24 lbs	195 lbs	25 lbs
$P_{total}$	2718 lbs	2787 lbs	2669 lbs	2879 lbs	3626 lbs	2841 lbs	828 lbs	780 lbs	782 lbs
$M$	70 lbs-ft	331 lbs-ft	75 lbs-ft	46 lbs-ft	251 lbs-ft	59 lbs-ft	72 lbs-ft	330 lbs-ft	74 lbs-ft
$e$	0.03 ft	0.12 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.09 ft	0.42 ft	0.10 ft
$L/6$	0.32 ft	1.68 ft	1.86 ft	1.88 ft	1.78 ft	1.88 ft	1.74 ft	1.07 ft	1.73 ft
$f_{min}$	260.7 sqft	182.8 sqft	253.8 sqft	285.3 sqft	296.6 sqft	277.2 sqft	62.9 sqft	-26.5 sqft	57.3 sqft
$f_{max}$	306.6 psf	398.8 psf	303.1 psf	315.6 psf	460.3 psf	315.6 psf	109.8 psf	189.3 psf	105.9 psf



Maximum Bearing Pressure = 460 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 23in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.384 k
Allowable Uplift =	1.214 k
Utilization =	<u>32%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.106 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.752 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>31%</u>

#### Diagonal Strut

Maximum Axial Load =	0.164 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.408 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

#### Bracing

Maximum Axial Load =	0.264 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.12 in
	<u>0.12 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 99.00 \text{ in}$$

$$J = 0.427$$

$$206.479$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 99.00 \text{ in}$$

$$J = 0.427$$

$$224.369$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.7 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.782 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.25 \\ &21.9891 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.7 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.25 \\ &24.5845 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.7 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

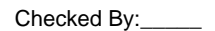
### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

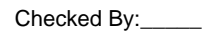
The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 15° 90mph 30psf 8.25ft 7-05.rdb Page 20



RISA-3D Version 13.0.0    \...\PVMini 60 Cell 1V 15° 90mph 30psf 8.25ft 7-05.r    Page 21





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-112.837	3	0	1	-34.95	4	0	1	-.012	4	0	1
87	6	max	586.528	1	0	1	-.116	12	0	1	0	12	0	1
88		min	-112.789	3	0	1	-35.006	4	0	1	-.016	4	0	1
89	7	max	586.593	1	0	1	-.116	12	0	1	0	12	0	1
90		min	-112.74	3	0	1	-35.062	4	0	1	-.019	4	0	1
91	8	max	586.658	1	0	1	-.116	12	0	1	0	12	0	1
92		min	-112.692	3	0	1	-35.118	4	0	1	-.022	4	0	1
93	9	max	586.722	1	0	1	-.116	12	0	1	0	12	0	1
94		min	-112.643	3	0	1	-35.174	4	0	1	-.025	4	0	1
95	10	max	586.787	1	0	1	-.116	12	0	1	0	12	0	1
96		min	-112.594	3	0	1	-35.23	4	0	1	-.028	4	0	1
97	11	max	586.852	1	0	1	-.116	12	0	1	0	12	0	1
98		min	-112.546	3	0	1	-35.286	4	0	1	-.031	4	0	1
99	12	max	586.916	1	0	1	-.116	12	0	1	0	12	0	1
100		min	-112.497	3	0	1	-35.342	4	0	1	-.034	4	0	1
101	13	max	586.981	1	0	1	-.116	12	0	1	0	12	0	1
102		min	-112.449	3	0	1	-35.398	4	0	1	-.038	4	0	1
103	14	max	587.046	1	0	1	-.116	12	0	1	0	12	0	1
104		min	-112.4	3	0	1	-35.454	4	0	1	-.041	4	0	1
105	15	max	587.11	1	0	1	-.116	12	0	1	0	12	0	1
106		min	-112.352	3	0	1	-35.51	4	0	1	-.044	4	0	1
107	16	max	587.175	1	0	1	-.116	12	0	1	0	12	0	1
108		min	-112.303	3	0	1	-35.566	4	0	1	-.047	4	0	1
109	17	max	587.24	1	0	1	-.116	12	0	1	0	12	0	1
110		min	-112.255	3	0	1	-35.623	4	0	1	-.05	4	0	1
111	18	max	587.305	1	0	1	-.116	12	0	1	0	12	0	1
112		min	-112.206	3	0	1	-35.679	4	0	1	-.054	4	0	1
113	19	max	587.369	1	0	1	-.116	12	0	1	0	12	0	1
114		min	-112.158	3	0	1	-35.735	4	0	1	-.057	4	0	1
115	M6	1	max	1405.827	1	.633	.987	4	0	1	0	5	0	1
116		min	-1207.953	3	.148	15	-.12	3	0	5	0	1	0	1
117	2	max	1405.923	1	.595	6	.9	4	0	1	0	4	0	15
118		min	-1207.881	3	.139	15	-.12	3	0	5	0	2	0	6
119	3	max	1406.02	1	.557	6	.813	4	0	1	0	4	0	15
120		min	-1207.809	3	.13	15	-.12	3	0	5	0	12	0	6
121	4	max	1406.116	1	.519	6	.725	4	0	1	0	4	0	15
122		min	-1207.737	3	.121	15	-.12	3	0	5	0	3	0	6
123	5	max	1406.213	1	.482	6	.638	4	0	1	0	4	0	15
124		min	-1207.664	3	.112	15	-.12	3	0	5	0	3	0	6
125	6	max	1406.309	1	.444	6	.55	4	0	1	0	4	0	15
126		min	-1207.592	3	.103	15	-.12	3	0	5	0	3	0	6
127	7	max	1406.405	1	.406	6	.463	4	0	1	0	4	0	15
128		min	-1207.52	3	.094	15	-.12	3	0	5	0	3	0	6
129	8	max	1406.502	1	.368	6	.4	14	0	1	0	4	0	15
130		min	-1207.447	3	.086	15	-.12	3	0	5	0	3	0	6
131	9	max	1406.598	1	.33	6	.356	14	0	1	0	4	0	15
132		min	-1207.375	3	.077	15	-.12	3	0	5	0	3	0	6
133	10	max	1406.694	1	.292	6	.337	1	0	1	0	4	0	15
134		min	-1207.303	3	.068	15	-.12	3	0	5	0	3	0	6
135	11	max	1406.791	1	.255	6	.337	1	0	1	0	4	0	15
136		min	-1207.231	3	.059	15	-.12	3	0	5	0	3	0	6
137	12	max	1406.887	1	.217	6	.337	1	0	1	0	4	0	15
138		min	-1207.158	3	.05	15	-.12	3	0	5	0	3	0	6
139	13	max	1406.983	1	.183	2	.337	1	0	1	0	4	0	15
140		min	-1207.086	3	.041	15	-.174	5	0	5	0	3	0	6
141	14	max	1407.08	1	.154	2	.337	1	0	1	0	4	0	15
142		min	-1207.014	3	.032	15	-.262	5	0	5	0	3	0	6





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1407.176	1	.124	2	.337	1	0	1	0	4	0	15
144		min	-1206.941	3	.008	9	-.349	5	0	5	0	3	0	6
145	16	max	1407.273	1	.097	10	.337	1	0	1	0	14	0	15
146		min	-1206.869	3	-.019	1	-.436	5	0	5	0	3	0	6
147	17	max	1407.369	1	.072	10	.337	1	0	1	0	14	0	15
148		min	-1206.797	3	-.048	1	-.524	5	0	5	0	3	0	6
149	18	max	1407.465	1	.048	10	.337	1	0	1	0	14	0	15
150		min	-1206.725	3	-.078	1	-.611	5	0	5	0	3	0	6
151	19	max	1407.562	1	.023	10	.337	1	0	1	0	14	0	15
152		min	-1206.652	3	-.107	1	-.698	5	0	5	0	3	0	6
153	M7	1	max	164.064	2	1.809	.015	1	0	2	0	4	0	4
154		min	-172.001	9	.43	15	-1.502	5	0	5	0	3	0	15
155	2	max	163.997	2	1.631	4	.015	1	0	2	0	4	0	2
156		min	-172.057	9	.388	15	-1.368	5	0	5	0	3	0	15
157	3	max	163.93	2	1.453	4	.015	1	0	2	0	4	0	2
158		min	-172.113	9	.346	15	-1.235	5	0	5	0	3	0	9
159	4	max	163.862	2	1.275	4	.015	1	0	2	0	14	0	10
160		min	-172.169	9	.304	15	-1.101	5	0	5	0	3	0	1
161	5	max	163.795	2	1.097	4	.015	1	0	2	0	2	0	15
162		min	-172.225	9	.262	15	-.968	5	0	5	0	5	0	1
163	6	max	163.728	2	.919	4	.015	1	0	2	0	2	0	15
164		min	-172.281	9	.22	15	-.834	5	0	5	0	5	0	6
165	7	max	163.661	2	.741	4	.015	1	0	2	0	2	0	15
166		min	-172.337	9	.179	15	-.701	5	0	5	0	5	0	6
167	8	max	163.594	2	.563	4	.015	1	0	2	0	2	0	15
168		min	-172.393	9	.137	15	-.567	5	0	5	0	5	0	6
169	9	max	163.527	2	.385	4	.015	1	0	2	0	2	0	15
170		min	-172.449	9	.095	15	-.434	5	0	5	0	5	-.001	6
171	10	max	163.46	2	.207	4	.015	1	0	2	0	2	0	15
172		min	-172.505	9	.053	15	-.3	5	0	5	0	5	-.001	6
173	11	max	163.393	2	.053	2	.015	1	0	2	0	2	0	15
174		min	-172.561	9	-.022	9	-.166	5	0	5	0	5	-.001	6
175	12	max	163.326	2	-.031	15	.015	1	0	2	0	2	0	15
176		min	-172.616	9	-.157	1	-.033	5	0	5	0	5	-.001	6
177	13	max	163.259	2	-.072	15	.103	4	0	2	0	2	0	15
178		min	-172.672	9	-.327	6	-.008	3	0	5	0	5	-.001	6
179	14	max	163.192	2	-.114	15	.237	4	0	2	0	2	0	15
180		min	-172.728	9	-.506	6	-.008	3	0	5	0	5	-.001	6
181	15	max	163.124	2	-.156	15	.37	4	0	2	0	2	0	15
182		min	-172.784	9	-.684	6	-.008	3	0	5	0	5	0	6
183	16	max	163.057	2	-.198	15	.504	4	0	2	0	2	0	15
184		min	-172.84	9	-.862	6	-.008	3	0	5	0	5	0	6
185	17	max	162.99	2	-.24	15	.637	4	0	2	0	2	0	15
186		min	-172.896	9	-1.04	6	-.008	3	0	5	0	5	0	6
187	18	max	162.923	2	-.282	15	.771	4	0	2	0	2	0	15
188		min	-172.952	9	-1.218	6	-.008	3	0	5	0	5	0	6
189	19	max	162.856	2	-.323	15	.904	4	0	2	0	2	0	1
190		min	-173.008	9	-1.396	6	-.008	3	0	5	0	5	0	1
191	M8	1	max	1751.006	1	0	.703	1	0	1	0	4	0	1
192		min	-394.232	3	0	1	-35.07	4	0	1	0	1	0	1
193	2	max	1751.07	1	0	1	.703	1	0	1	0	1	0	1
194		min	-394.183	3	0	1	-35.126	4	0	1	-.003	4	0	1
195	3	max	1751.135	1	0	1	.703	1	0	1	0	1	0	1
196		min	-394.135	3	0	1	-35.182	4	0	1	-.006	4	0	1
197	4	max	1751.2	1	0	1	.703	1	0	1	0	1	0	1
198		min	-394.086	3	0	1	-35.238	4	0	1	-.009	4	0	1
199	5	max	1751.265	1	0	1	.703	1	0	1	0	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	439.677	1	.151	4	.015	4	.001	1	.001	4	0	15
258		min	-357.61	3	-.004	1	-.118	1	-.002	5	0	1	0	4
259	16	max	439.773	1	.113	4	-.026	12	.001	1	.001	4	0	15
260		min	-357.538	3	-.034	1	-.118	1	-.002	5	0	1	0	4
261	17	max	439.87	1	.075	4	-.026	12	.001	1	.001	4	0	15
262		min	-357.466	3	-.063	1	-.172	5	-.002	5	0	1	0	4
263	18	max	439.966	1	.053	3	-.026	12	.001	1	.001	4	0	15
264		min	-357.393	3	-.093	1	-.26	5	-.002	5	0	1	0	4
265	19	max	440.062	1	.031	3	-.026	12	.001	1	.001	4	0	15
266		min	-357.321	3	-.122	1	-.347	5	-.002	5	0	1	0	4
267	M11	1	max	36.185	10	1.806	.874	1	.002	4	.002	5	0	6
268		min	-123.676	1	.423	15	-1.114	5	0	10	-.002	1	0	15
269	2	max	36.13	10	1.628	6	.874	1	.002	4	.001	5	0	2
270		min	-123.743	1	.382	15	-.981	5	0	10	-.002	1	0	15
271	3	max	36.074	10	1.45	6	.874	1	.002	4	.001	5	0	2
272		min	-123.81	1	.34	15	-.847	5	0	10	-.002	1	0	3
273	4	max	36.018	10	1.272	6	.874	1	.002	4	.001	5	0	15
274		min	-123.877	1	.298	15	-.714	5	0	10	-.002	1	0	4
275	5	max	35.962	10	1.094	6	.874	1	.002	4	0	5	0	15
276		min	-123.944	1	.256	15	-.58	5	0	10	-.001	1	0	4
277	6	max	35.906	10	.916	6	.874	1	.002	4	0	5	0	15
278		min	-124.012	1	.214	15	-.447	5	0	10	-.001	1	0	4
279	7	max	35.85	10	.738	6	.874	1	.002	4	0	5	0	15
280		min	-124.079	1	.172	15	-.313	5	0	10	0	1	0	4
281	8	max	35.794	10	.56	6	.874	1	.002	4	0	5	0	15
282		min	-124.146	1	.131	15	-.18	5	0	10	0	1	0	4
283	9	max	35.738	10	.382	6	.874	1	.002	4	0	5	0	15
284		min	-124.213	1	.089	15	-.046	5	0	10	0	1	-.001	4
285	10	max	35.682	10	.204	6	.874	1	.002	4	0	5	0	15
286		min	-124.28	1	.047	15	.02	12	0	10	0	1	-.001	4
287	11	max	35.626	10	.051	2	.874	1	.002	4	0	5	0	15
288		min	-124.347	1	.002	3	.02	12	0	10	0	1	-.001	4
289	12	max	35.57	10	-.037	15	.874	1	.002	4	0	5	0	15
290		min	-124.414	1	-.152	4	.02	12	0	10	0	2	-.001	4
291	13	max	35.514	10	-.079	15	.874	1	.002	4	0	4	0	15
292		min	-124.481	1	-.33	4	.02	12	0	10	0	2	-.001	4
293	14	max	35.459	10	-.121	15	.874	1	.002	4	0	4	0	15
294		min	-124.548	1	-.508	4	.02	12	0	10	0	10	-.001	4
295	15	max	35.403	10	-.162	15	.921	4	.002	4	.001	4	0	15
296		min	-124.615	1	-.686	4	.02	12	0	10	0	10	0	4
297	16	max	35.347	10	-.204	15	1.055	4	.002	4	.001	4	0	15
298		min	-124.682	1	-.864	4	.02	12	0	10	0	10	0	4
299	17	max	35.291	10	-.246	15	1.188	4	.002	4	.002	4	0	15
300		min	-124.75	1	-1.042	4	.02	12	0	10	0	10	0	4
301	18	max	35.235	10	-.288	15	1.322	4	.002	4	.002	4	0	15
302		min	-124.817	1	-1.22	4	.02	12	0	10	0	10	0	4
303	19	max	35.179	10	-.33	15	1.456	4	.002	4	.002	4	0	1
304		min	-124.884	1	-1.398	4	.02	12	0	10	0	10	0	1
305	M12	1	max	586.105	1	0	1	3.487	1	0	1	0	4	1
306		min	-112.627	3	0	1	-31.992	5	0	1	0	3	0	1
307	2	max	586.17	1	0	1	3.487	1	0	1	0	1	0	1
308		min	-112.578	3	0	1	-32.048	5	0	1	-.003	5	0	1
309	3	max	586.235	1	0	1	3.487	1	0	1	0	1	0	1
310		min	-112.53	3	0	1	-32.104	5	0	1	-.006	5	0	1
311	4	max	586.299	1	0	1	3.487	1	0	1	0	1	0	1
312		min	-112.481	3	0	1	-32.161	5	0	1	-.009	5	0	1
313	5	max	586.364	1	0	1	3.487	1	0	1	.001	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-112.433	3	0	1	-32.217	5	0	1	-.011	5	0	1
315		6	max	586.429	1	0	1	3.487	1	0	1	.002	1	0	1
316			min	-112.384	3	0	1	-32.273	5	0	1	-.014	5	0	1
317		7	max	586.494	1	0	1	3.487	1	0	1	.002	1	0	1
318			min	-112.336	3	0	1	-32.329	5	0	1	-.017	5	0	1
319		8	max	586.558	1	0	1	3.487	1	0	1	.002	1	0	1
320			min	-112.287	3	0	1	-32.385	5	0	1	-.02	5	0	1
321		9	max	586.623	1	0	1	3.487	1	0	1	.003	1	0	1
322			min	-112.239	3	0	1	-32.441	5	0	1	-.023	5	0	1
323		10	max	586.688	1	0	1	3.487	1	0	1	.003	1	0	1
324			min	-112.19	3	0	1	-32.497	5	0	1	-.026	5	0	1
325		11	max	586.752	1	0	1	3.487	1	0	1	.003	1	0	1
326			min	-112.142	3	0	1	-32.553	5	0	1	-.029	5	0	1
327		12	max	586.817	1	0	1	3.487	1	0	1	.003	1	0	1
328			min	-112.093	3	0	1	-32.609	5	0	1	-.032	5	0	1
329		13	max	586.882	1	0	1	3.487	1	0	1	.004	1	0	1
330			min	-112.045	3	0	1	-32.665	5	0	1	-.035	5	0	1
331		14	max	586.947	1	0	1	3.487	1	0	1	.004	1	0	1
332			min	-111.996	3	0	1	-32.721	5	0	1	-.038	5	0	1
333		15	max	587.011	1	0	1	3.487	1	0	1	.004	1	0	1
334			min	-111.948	3	0	1	-32.777	5	0	1	-.041	5	0	1
335		16	max	587.076	1	0	1	3.487	1	0	1	.005	1	0	1
336			min	-111.899	3	0	1	-32.834	5	0	1	-.043	5	0	1
337		17	max	587.141	1	0	1	3.487	1	0	1	.005	1	0	1
338			min	-111.851	3	0	1	-32.89	5	0	1	-.046	5	0	1
339		18	max	587.205	1	0	1	3.487	1	0	1	.005	1	0	1
340			min	-111.802	3	0	1	-32.946	5	0	1	-.049	5	0	1
341		19	max	587.27	1	0	1	3.487	1	0	1	.006	1	0	1
342			min	-111.753	3	0	1	-33.002	5	0	1	-.052	5	0	1
343	M1	1	max	114.791	1	344.851	3	-2.428	12	0	1	.135	1	.015	1
344			min	3.824	12	-427.406	1	-68.456	1	0	3	.005	12	-.01	3
345		2	max	114.863	1	344.648	3	-2.428	12	0	1	.12	1	.108	1
346			min	3.86	12	-427.676	1	-68.456	1	0	3	.005	12	-.085	3
347		3	max	130.336	1	7.002	9	-2.462	12	0	5	.104	1	.199	1
348			min	-6.681	3	-23.433	3	-68.042	1	0	1	.004	12	-.158	3
349		4	max	130.409	1	6.777	9	-2.462	12	0	5	.089	1	.199	1
350			min	-6.627	3	-23.635	3	-68.042	1	0	1	.004	12	-.153	3
351		5	max	130.481	1	6.553	9	-2.462	12	0	5	.075	1	.199	1
352			min	-6.573	3	-23.838	3	-68.042	1	0	1	.003	12	-.148	3
353		6	max	130.553	1	6.328	9	-2.462	12	0	5	.06	1	.199	1
354			min	-6.519	3	-24.04	3	-68.042	1	0	1	.002	12	-.143	3
355		7	max	130.625	1	6.103	9	-2.462	12	0	5	.045	1	.199	1
356			min	-6.464	3	-24.242	3	-68.042	1	0	1	.002	12	-.137	3
357		8	max	130.698	1	5.878	9	-2.462	12	0	5	.03	1	.199	1
358			min	-6.41	3	-24.445	3	-68.042	1	0	1	.001	12	-.132	3
359		9	max	130.77	1	5.654	9	-2.462	12	0	5	.016	1	.199	1
360			min	-6.356	3	-24.647	3	-68.042	1	0	1	0	12	-.127	3
361		10	max	130.842	1	5.429	9	-2.462	12	0	5	.003	4	.199	1
362			min	-6.302	3	-24.849	3	-68.042	1	0	1	0	10	-.121	3
363		11	max	130.914	1	5.204	9	-2.462	12	0	5	0	15	.2	1
364			min	-6.248	3	-25.052	3	-68.042	1	0	1	-.014	1	-.116	3
365		12	max	130.987	1	4.979	9	-2.462	12	0	5	0	12	.2	1
366			min	-6.193	3	-25.254	3	-68.042	1	0	1	-.029	1	-.111	3
367		13	max	131.059	1	4.755	9	-2.462	12	0	5	-.001	12	.2	1
368			min	-6.139	3	-25.456	3	-68.042	1	0	1	-.043	1	-.105	3
369		14	max	131.131	1	4.53	9	-2.462	12	0	5	-.002	12	.201	1
370			min	-6.085	3	-25.658	3	-68.042	1	0	1	-.058	1	-.1	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	131.204	1	4.305	9	-2.462	12	0	5	-.002	12	.202	1
372		min	-6.031	3	-25.861	3	-68.042	1	0	1	-.073	1	-.094	3
373	16	max	69.1	2	9.957	10	-2.493	12	0	1	-.003	12	.203	1
374		min	-34.682	3	-85.41	1	-68.7	1	0	4	-.089	1	-.088	3
375	17	max	69.172	2	9.732	10	-2.493	12	0	1	-.003	12	.221	1
376		min	-34.627	3	-85.679	1	-68.7	1	0	4	-.104	1	-.077	3
377	18	max	-3.559	12	479.405	1	-2.61	12	0	5	-.004	12	.12	1
378		min	-114.403	1	-161.492	3	-70.297	1	0	1	-.119	1	-.042	3
379	19	max	-3.523	12	479.135	1	-2.61	12	0	5	-.005	12	.016	1
380		min	-114.331	1	-161.695	3	-70.297	1	0	1	-.134	1	-.007	3
381	M5	1	max	251.888	1	1140.143	3	-.068	10	0	.049	4	.02	3
382		min	4.769	15	-1413.652	1	-29.891	4	0	5	0	10	-.03	1
383	2	max	251.96	1	1139.94	3	-.068	10	0	1	.042	4	.277	1
384		min	4.791	15	-1413.922	1	-29.649	4	0	5	-.002	3	-.227	3
385	3	max	301.193	1	10.416	9	2.628	3	0	3	.036	4	.578	1
386		min	-32.025	3	-77.11	3	-26.302	4	0	4	-.007	3	-.469	3
387	4	max	301.265	1	10.191	9	2.628	3	0	3	.03	4	.581	1
388		min	-31.97	3	-77.312	3	-26.06	4	0	4	-.007	3	-.452	3
389	5	max	301.337	1	9.966	9	2.628	3	0	3	.024	4	.584	1
390		min	-31.916	3	-77.514	3	-25.818	4	0	4	-.006	3	-.436	3
391	6	max	301.409	1	9.742	9	2.628	3	0	3	.019	4	.588	1
392		min	-31.862	3	-77.716	3	-25.576	4	0	4	-.005	3	-.419	3
393	7	max	301.482	1	9.517	9	2.628	3	0	3	.013	4	.591	1
394		min	-31.808	3	-77.919	3	-25.334	4	0	4	-.005	3	-.402	3
395	8	max	301.554	1	9.292	9	2.628	3	0	3	.008	4	.595	1
396		min	-31.754	3	-78.121	3	-25.092	4	0	4	-.004	3	-.385	3
397	9	max	301.626	1	9.067	9	2.628	3	0	3	.002	5	.598	1
398		min	-31.699	3	-78.323	3	-24.85	4	0	4	-.004	3	-.368	3
399	10	max	301.699	1	8.843	9	2.628	3	0	3	0	10	.602	1
400		min	-31.645	3	-78.526	3	-24.608	4	0	4	-.003	3	-.351	3
401	11	max	301.771	1	8.618	9	2.628	3	0	3	0	10	.605	1
402		min	-31.591	3	-78.728	3	-24.366	4	0	4	-.008	4	-.334	3
403	12	max	301.843	1	8.393	9	2.628	3	0	3	0	10	.609	1
404		min	-31.537	3	-78.93	3	-24.124	4	0	4	-.014	4	-.317	3
405	13	max	301.915	1	8.168	9	2.628	3	0	3	0	10	.613	1
406		min	-31.483	3	-79.132	3	-23.882	4	0	4	-.019	4	-.3	3
407	14	max	301.988	1	7.944	9	2.628	3	0	3	0	10	.616	1
408		min	-31.428	3	-79.335	3	-23.64	4	0	4	-.024	4	-.283	3
409	15	max	302.06	1	7.719	9	2.628	3	0	3	0	10	.62	1
410		min	-31.374	3	-79.537	3	-23.398	4	0	4	-.029	4	-.265	3
411	16	max	256.161	2	54.25	10	2.607	3	0	1	0	3	.625	1
412		min	-112.685	3	-150.197	3	-22.264	4	0	4	-.035	4	-.247	3
413	17	max	256.233	2	54.026	10	2.607	3	0	1	0	3	.647	1
414		min	-112.631	3	-150.4	3	-22.022	4	0	4	-.039	4	-.215	3
415	18	max	-7.244	12	1580.313	1	2.392	3	0	4	.001	3	.311	1
416		min	-252.583	1	-532.2	3	-55.391	5	0	1	-.051	4	-.101	3
417	19	max	-7.208	12	1580.044	1	2.392	3	0	4	.002	3	.014	3
418		min	-252.511	1	-532.403	3	-55.149	5	0	1	-.063	4	-.031	1
419	M9	1	max	114.267	1	344.84	3	225.447	4	0	3	0	.015	1
420		min	1.564	15	-427.389	1	5.382	10	0	1	-.135	1	-.01	3
421	2	max	114.34	1	344.637	3	225.689	4	0	3	.046	5	.108	1
422		min	1.586	15	-427.659	1	5.382	10	0	1	-.115	1	-.085	3
423	3	max	130.41	1	6.982	9	64.15	1	0	1	.088	5	.198	1
424		min	-6.261	3	-23.378	3	-35.373	5	0	12	-.094	1	-.158	3
425	4	max	130.482	1	6.757	9	64.15	1	0	1	.081	5	.198	1
426		min	-6.207	3	-23.581	3	-35.131	5	0	12	-.08	1	-.153	3
427	5	max	130.555	1	6.532	9	64.15	1	0	1	.073	5	.198	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-6.153	3	-23.783	3	-34.889	5	0	12	-.066	1	-.148	3
429		6	max	130.627	1	6.307	9	64.15	1	0	1	.066	5	.198	1
430			min	-6.099	3	-23.985	3	-34.647	5	0	12	-.052	1	-.143	3
431		7	max	130.699	1	6.083	9	64.15	1	0	1	.058	5	.199	1
432			min	-6.044	3	-24.188	3	-34.405	5	0	12	-.038	1	-.137	3
433		8	max	130.771	1	5.858	9	64.15	1	0	1	.051	5	.199	1
434			min	-5.99	3	-24.39	3	-34.163	5	0	12	-.025	1	-.132	3
435		9	max	130.844	1	5.633	9	64.15	1	0	1	.043	5	.199	1
436			min	-5.936	3	-24.592	3	-33.921	5	0	12	-.011	1	-.127	3
437		10	max	130.916	1	5.408	9	64.15	1	0	1	.036	4	.199	1
438			min	-5.882	3	-24.795	3	-33.679	5	0	12	0	2	-.121	3
439		11	max	130.988	1	5.184	9	64.15	1	0	1	.032	4	.2	1
440			min	-5.828	3	-24.997	3	-33.437	5	0	12	.001	10	-.116	3
441		12	max	131.06	1	4.959	9	64.15	1	0	1	.031	1	.2	1
442			min	-5.773	3	-25.199	3	-33.195	5	0	12	.002	10	-.111	3
443		13	max	131.133	1	4.734	9	64.15	1	0	1	.045	1	.2	1
444			min	-5.719	3	-25.401	3	-32.953	5	0	12	.003	12	-.105	3
445		14	max	131.205	1	4.509	9	64.15	1	0	1	.059	1	.201	1
446			min	-5.665	3	-25.604	3	-32.711	5	0	12	.003	12	-.1	3
447		15	max	131.277	1	4.285	9	64.15	1	0	1	.073	1	.202	1
448			min	-5.611	3	-25.806	3	-32.469	5	0	12	0	15	-.094	3
449		16	max	69.303	2	9.603	10	64.969	1	0	10	.088	1	.203	1
450			min	-34.75	3	-85.314	1	-30.945	5	0	4	-.004	5	-.088	3
451		17	max	69.376	2	9.378	10	64.969	1	0	10	.103	1	.221	1
452			min	-34.695	3	-85.584	1	-30.703	5	0	4	-.011	5	-.077	3
453		18	max	4.016	5	479.405	1	68.357	1	0	1	.117	1	.12	1
454			min	-114.177	1	-161.491	3	-62.357	5	0	3	-.024	5	-.042	3
455		19	max	4.05	5	479.135	1	68.357	1	0	1	.132	1	.016	1
456			min	-114.105	1	-161.693	3	-62.115	5	0	3	-.038	5	-.007	3
457	M13	1	max	225.454	4	426.817	1	-1.564	15	.015	1	.135	1	0	1
458			min	5.383	10	-344.83	3	-114.255	1	-.01	3	0	15	0	3
459		2	max	216.378	4	301.091	1	-.746	15	.015	1	.042	1	.269	3
460			min	5.383	10	-243.181	3	-87.536	1	-.01	3	-.002	5	-.334	1
461		3	max	207.301	4	175.365	1	.072	15	.015	1	.001	3	.446	3
462			min	5.383	10	-141.533	3	-60.817	1	-.01	3	-.026	1	-.552	1
463		4	max	198.225	4	49.638	1	1.266	5	.015	1	0	12	.529	3
464			min	5.383	10	-39.884	3	-34.097	1	-.01	3	-.069	1	-.655	1
465		5	max	189.148	4	61.765	3	2.531	5	.015	1	0	15	.519	3
466			min	5.383	10	-76.088	1	-7.378	1	-.01	3	-.088	1	-.643	1
467		6	max	180.072	4	163.413	3	19.341	1	.015	1	.003	5	.416	3
468			min	5.383	10	-201.814	1	.254	12	-.01	3	-.083	1	-.516	1
469		7	max	170.996	4	265.062	3	46.061	1	.015	1	.007	5	.219	3
470			min	5.383	10	-327.54	1	1.051	12	-.01	3	-.053	1	-.273	1
471		8	max	161.919	4	366.71	3	72.78	1	.015	1	.013	4	.085	1
472			min	5.383	10	-453.267	1	1.849	12	-.01	3	0	12	-.07	3
473		9	max	152.843	4	468.359	3	99.499	1	.015	1	.08	1	.558	1
474			min	5.383	10	-578.993	1	2.647	12	-.01	3	.002	12	-.453	3
475		10	max	143.766	4	570.008	3	126.219	1	.011	2	.184	1	1.146	1
476			min	5.383	10	-704.719	1	3.445	12	-.015	1	.005	12	-.929	3
477		11	max	104.908	4	578.993	1	2.565	5	.01	3	.077	1	.558	1
478			min	2.428	12	-468.359	3	-98.973	1	-.015	1	-.019	5	-.453	3
479		12	max	95.831	4	453.267	1	3.83	5	.01	3	0	2	.085	1
480			min	2.428	12	-366.71	3	-72.253	1	-.015	1	-.017	4	-.07	3
481		13	max	86.755	4	327.54	1	5.095	5	.01	3	-.003	12	.219	3
482			min	2.428	12	-265.062	3	-45.534	1	-.015	1	-.056	1	-.273	1
483		14	max	77.678	4	201.814	1	6.359	5	.01	3	-.003	12	.416	3
484			min	2.428	12	-163.413	3	-18.815	1	-.015	1	-.085	1	-.516	1



Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	68.646	1	76.088	1	9.288	4	.01	3	0	15	.519	3
486		min	2.428	12	-61.765	3	.399	10	-.015	1	-.09	1	-.643	1
487	16	max	68.646	1	39.884	3	34.624	1	.01	3	.007	5	.529	3
488		min	2.428	12	-49.639	1	1.431	12	-.015	1	-.071	1	-.655	1
489	17	max	68.646	1	141.533	3	61.343	1	.01	3	.016	5	.446	3
490		min	2.428	12	-175.365	1	2.229	12	-.015	1	-.027	1	-.552	1
491	18	max	68.646	1	243.181	3	88.063	1	.01	3	.042	1	.269	3
492		min	2.428	12	-301.091	1	3.026	12	-.015	1	.002	12	-.334	1
493	19	max	68.646	1	344.83	3	114.782	1	.01	3	.135	1	0	1
494		min	2.428	12	-426.818	1	3.824	12	-.015	1	.005	12	0	3
495	M16	1	max	62.102	5	479.74	1	4.05	.007	3	.132	1	0	1
496		min	-68.151	1	-161.709	3	-114.115	1	-.016	1	-.038	5	0	3
497	2	max	53.025	5	338.409	1	5.314	5	.007	3	.04	1	.126	3
498		min	-68.151	1	-114.158	3	-87.396	1	-.016	1	-.034	5	-.375	1
499	3	max	43.949	5	197.077	1	6.579	5	.007	3	0	12	.209	3
500		min	-68.151	1	-66.608	3	-60.676	1	-.016	1	-.034	4	-.62	1
501	4	max	34.872	5	55.745	1	7.844	5	.007	3	-.002	12	.249	3
502		min	-68.151	1	-19.057	3	-33.957	1	-.016	1	-.071	1	-.736	1
503	5	max	25.796	5	28.494	3	9.109	5	.007	3	-.003	12	.244	3
504		min	-68.151	1	-85.586	1	-7.238	1	-.016	1	-.09	1	-.723	1
505	6	max	16.719	5	76.044	3	19.482	1	.007	3	-.003	12	.196	3
506		min	-68.151	1	-226.918	1	.359	12	-.016	1	-.085	1	-.579	1
507	7	max	7.643	5	123.595	3	46.201	1	.007	3	.005	5	.105	3
508		min	-68.151	1	-368.25	1	1.157	12	-.016	1	-.055	1	-.307	1
509	8	max	-.898	15	171.145	3	72.92	1	.007	3	.017	4	.096	1
510		min	-68.151	1	-509.581	1	1.955	12	-.016	1	-.001	3	-.03	3
511	9	max	-1.109	12	218.696	3	99.64	1	.007	3	.079	1	.628	1
512		min	-68.151	1	-650.913	1	2.753	12	-.016	1	.001	12	-.209	3
513	10	max	35.586	5	-17.102	15	126.359	1	.006	14	.183	1	1.289	1
514		min	-70.115	1	-792.245	1	-5.477	3	-.016	1	.005	12	-.431	3
515	11	max	26.509	5	650.913	1	2.621	5	.016	1	.079	1	.628	1
516		min	-70.115	1	-218.696	3	-99.413	1	-.007	3	-.018	5	-.209	3
517	12	max	17.433	5	509.581	1	3.886	5	.016	1	0	2	.096	1
518		min	-70.115	1	-171.145	3	-72.694	1	-.007	3	-.015	4	-.03	3
519	13	max	8.356	5	368.25	1	5.151	5	.016	1	-.002	12	.105	3
520		min	-70.115	1	-123.595	3	-45.975	1	-.007	3	-.054	1	-.307	1
521	14	max	-.391	15	226.918	1	6.415	5	.016	1	-.002	12	.196	3
522		min	-70.115	1	-76.044	3	-19.255	1	-.007	3	-.084	1	-.579	1
523	15	max	-2.61	12	85.586	1	9.315	4	.016	1	.001	5	.244	3
524		min	-70.115	1	-28.494	3	.331	12	-.007	3	-.089	1	-.723	1
525	16	max	-2.61	12	19.057	3	34.183	1	.016	1	.009	5	.249	3
526		min	-70.115	1	-55.745	1	1.129	12	-.007	3	-.07	1	-.736	1
527	17	max	-2.61	12	66.608	3	60.903	1	.016	1	.018	5	.209	3
528		min	-70.115	1	-197.077	1	1.927	12	-.007	3	-.027	1	-.62	1
529	18	max	-2.61	12	114.158	3	87.622	1	.016	1	.042	1	.126	3
530		min	-70.115	1	-338.409	1	2.725	12	-.007	3	.002	12	-.375	1
531	19	max	-2.61	12	161.709	3	114.341	1	.016	1	.134	1	0	1
532		min	-70.115	1	-479.74	1	3.523	12	-.007	3	.005	12	0	5
533	M15	1	max	.348	2	2.236	1	.023	3	0	1	0	1	1
534		min	-26.423	3	0	4	-.031	1	0	3	0	3	0	1
535	2	max	.276	2	1.988	1	.023	3	0	1	0	1	0	4
536		min	-26.477	3	0	4	-.031	1	0	3	0	3	-.001	1
537	3	max	.204	2	1.739	1	.023	3	0	1	0	1	0	4
538		min	-26.531	3	0	4	-.031	1	0	3	0	3	-.002	1
539	4	max	.132	2	1.491	1	.023	3	0	1	0	1	0	4
540		min	-26.585	3	0	4	-.031	1	0	3	0	3	-.003	1
541	5	max	.06	2	1.242	1	.023	3	0	1	0	1	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-26.639	3	0	4	-.031	1	0	3	0	3	-.003	1
543		6	max	0	4	.994	1	.023	3	0	1	0	1	0	4
544			min	-26.693	3	0	4	-.031	1	0	3	0	3	-.004	1
545		7	max	0	4	.745	1	.023	3	0	1	0	3	0	4
546			min	-26.747	3	0	4	-.031	1	0	3	0	1	-.004	1
547		8	max	0	4	.497	1	.023	3	0	1	0	3	0	4
548			min	-26.801	3	0	4	-.031	1	0	3	0	1	-.005	1
549		9	max	0	4	.248	1	.023	3	0	1	0	3	0	4
550			min	-26.855	3	0	4	-.031	1	0	3	0	1	-.005	1
551		10	max	0	4	0	1	.023	3	0	1	0	3	0	4
552			min	-26.909	3	0	1	-.031	1	0	3	0	1	-.005	1
553		11	max	0	4	0	4	.023	3	0	1	0	3	0	4
554			min	-26.963	3	-.248	2	-.031	1	0	3	0	1	-.005	1
555		12	max	0	4	0	4	.023	3	0	1	0	3	0	4
556			min	-27.017	3	-.497	2	-.031	1	0	3	0	1	-.005	1
557		13	max	0	4	0	4	.023	3	0	1	0	3	0	4
558			min	-27.071	3	-.745	2	-.031	1	0	3	0	1	-.004	1
559		14	max	0	4	0	4	.023	3	0	1	0	3	0	4
560			min	-27.125	3	-.994	2	-.031	1	0	3	0	1	-.004	1
561		15	max	0	4	0	4	.023	3	0	1	0	3	0	4
562			min	-27.179	3	-1.242	2	-.031	1	0	3	0	1	-.003	1
563		16	max	0	4	0	4	.023	3	0	1	0	3	0	4
564			min	-27.233	3	-1.491	2	-.031	1	0	3	0	1	-.003	1
565		17	max	0	4	0	4	.023	3	0	1	0	3	0	4
566			min	-27.287	3	-1.739	2	-.031	1	0	3	0	1	-.002	1
567		18	max	0	4	0	4	.023	3	0	1	0	3	0	4
568			min	-27.341	3	-1.988	2	-.031	1	0	3	0	1	-.001	1
569		19	max	0	4	0	4	.023	3	0	1	0	3	0	1
570			min	-27.395	3	-2.236	2	-.031	1	0	3	0	1	0	1
571	M16A	1	max	-.747	10	3.429	4	.208	4	0	3	0	3	0	1
572			min	-262.206	4	1.051	15	-.009	3	0	1	0	4	0	1
573		2	max	-.687	10	3.048	4	.188	4	0	3	0	3	0	15
574			min	-262.328	4	.934	15	-.009	3	0	1	0	4	-.002	4
575		3	max	-.627	10	2.667	4	.168	4	0	3	0	3	0	15
576			min	-262.45	4	.817	15	-.009	3	0	1	0	4	-.003	4
577		4	max	-.567	10	2.286	4	.148	4	0	3	0	3	-.001	15
578			min	-262.572	4	.701	15	-.009	3	0	1	0	4	-.004	4
579		5	max	-.507	10	1.905	4	.129	4	0	3	0	3	-.002	15
580			min	-262.694	4	.584	15	-.009	3	0	1	0	1	-.005	4
581		6	max	-.447	10	1.524	4	.109	4	0	3	0	5	-.002	15
582			min	-262.816	4	.467	15	-.009	3	0	1	0	1	-.006	4
583		7	max	-.387	10	1.143	4	.089	4	0	3	0	5	-.002	15
584			min	-262.939	4	.35	15	-.009	3	0	1	0	1	-.007	4
585		8	max	-.327	10	.762	4	.069	4	0	3	0	5	-.002	15
586			min	-263.061	4	.234	15	-.009	3	0	1	0	1	-.007	4
587		9	max	-.267	10	.381	4	.049	4	0	3	0	5	-.002	15
588			min	-263.183	4	.117	15	-.009	3	0	1	0	1	-.007	4
589		10	max	-.207	10	0	1	.029	4	0	3	0	5	-.002	15
590			min	-263.305	4	0	1	-.009	3	0	1	0	1	-.007	4
591		11	max	-.147	10	-.117	15	.019	1	0	3	0	5	-.002	15
592			min	-263.427	4	-.381	4	-.009	3	0	1	0	1	-.007	4
593		12	max	-.087	10	-.234	15	.019	1	0	3	0	5	-.002	15
594			min	-263.549	4	-.762	4	-.014	5	0	1	0	1	-.007	4
595		13	max	-.027	10	-.35	15	.019	1	0	3	0	5	-.002	15
596			min	-263.671	4	-1.143	4	-.034	5	0	1	0	3	-.007	4
597		14	max	.033	10	-.467	15	.019	1	0	3	0	4	-.002	15
598			min	-263.794	4	-1.524	4	-.054	5	0	1	0	3	-.006	4





RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 15° 90mph 30psf 8.25ft 7-05.rdb Page 32



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	1	0	2	.012	5	2.019e-4	1	NC	1	NC	1
44			min	0	10	-.001	3	0	1	-1.119e-3	5	NC	1	8189.518	14
45		4	max	0	1	0	2	.018	5	2.375e-4	1	NC	1	NC	1
46			min	0	10	-.002	3	-.001	1	-1.124e-3	5	NC	1	5354.205	14
47		5	max	0	1	0	2	.024	4	2.732e-4	1	NC	1	NC	1
48			min	0	10	-.003	3	-.001	1	-1.13e-3	5	NC	1	3948.298	14
49		6	max	0	1	0	2	.03	4	3.089e-4	1	NC	1	NC	1
50			min	0	10	-.003	3	-.001	1	-1.136e-3	5	NC	1	3112.949	14
51		7	max	0	1	0	2	.036	4	3.446e-4	1	NC	1	NC	1
52			min	0	10	-.004	3	0	1	-1.141e-3	5	NC	1	2562.042	14
53		8	max	0	1	.001	2	.042	4	3.802e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	-1.147e-3	5	NC	1	2173.066	14
55		9	max	0	1	.002	2	.048	4	4.159e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	2	-1.152e-3	5	NC	1	1884.843	14
57		10	max	0	1	.002	2	.054	4	4.516e-4	1	NC	1	NC	1
58			min	0	10	-.006	3	0	10	-1.158e-3	5	NC	1	1663.438	14
59		11	max	0	1	.003	2	.06	4	4.872e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	10	-1.164e-3	5	NC	1	1488.529	14
61		12	max	0	1	.003	2	.065	4	5.229e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	12	-1.169e-3	5	NC	1	1347.211	14
63		13	max	0	1	.004	2	.071	4	5.586e-4	1	NC	1	NC	1
64			min	0	10	-.007	3	0	12	-1.175e-3	5	NC	1	1230.898	14
65		14	max	.001	1	.005	2	.077	4	5.942e-4	1	NC	3	NC	1
66			min	0	10	-.007	3	0	12	-1.18e-3	5	9619.549	2	1133.664	14
67		15	max	.001	1	.006	2	.083	4	6.299e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	12	-1.186e-3	5	8122.842	2	1051.282	14
69		16	max	.001	1	.007	2	.088	4	6.656e-4	1	NC	3	NC	2
70			min	0	10	-.007	3	0	12	-1.192e-3	5	6955.762	2	980.661	14
71		17	max	.001	1	.008	1	.094	4	7.012e-4	1	NC	3	NC	2
72			min	0	10	-.007	3	0	12	-1.197e-3	5	6022.417	1	919.486	14
73		18	max	.001	1	.009	1	.099	4	7.369e-4	1	NC	3	NC	2
74			min	0	10	-.007	3	0	12	-1.203e-3	5	5269.378	1	865.99	14
75		19	max	.001	1	.01	1	.105	4	7.726e-4	1	NC	3	NC	2
76			min	0	10	-.007	3	0	12	-1.208e-3	5	4673.114	1	818.799	14
77	M4	1	max	.003	1	.007	2	0	12	4.558e-3	5	NC	1	NC	2
78			min	0	3	-.005	3	-.111	4	-8.691e-4	1	NC	1	174.436	4
79		2	max	.003	1	.007	2	0	12	4.558e-3	5	NC	1	NC	2
80			min	0	3	-.005	3	-.102	4	-8.691e-4	1	NC	1	190.164	4
81		3	max	.002	1	.007	2	0	12	4.558e-3	5	NC	1	NC	2
82			min	0	3	-.005	3	-.093	4	-8.691e-4	1	NC	1	208.884	4
83		4	max	.002	1	.006	2	0	12	4.558e-3	5	NC	1	NC	2
84			min	0	3	-.004	3	-.084	4	-8.691e-4	1	NC	1	231.384	4
85		5	max	.002	1	.006	2	0	12	4.558e-3	5	NC	1	NC	2
86			min	0	3	-.004	3	-.075	4	-8.691e-4	1	NC	1	258.74	4
87		6	max	.002	1	.005	2	0	12	4.558e-3	5	NC	1	NC	2
88			min	0	3	-.004	3	-.066	4	-8.691e-4	1	NC	1	292.443	4
89		7	max	.002	1	.005	2	0	12	4.558e-3	5	NC	1	NC	2
90			min	0	3	-.004	3	-.058	4	-8.691e-4	1	NC	1	334.622	4
91		8	max	.002	1	.004	2	0	12	4.558e-3	5	NC	1	NC	2
92			min	0	3	-.003	3	-.05	4	-8.691e-4	1	NC	1	388.391	4
93		9	max	.002	1	.004	2	0	12	4.558e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.042	4	-8.691e-4	1	NC	1	458.459	4
95		10	max	.001	1	.004	2	0	12	4.558e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.035	4	-8.691e-4	1	NC	1	552.234	4
97		11	max	.001	1	.003	2	0	12	4.558e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.028	4	-8.691e-4	1	NC	1	681.911	4
99		12	max	.001	1	.003	2	0	12	4.558e-3	5	NC	1	NC	1

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.002	3	-.022	4	-8.691e-4	1	NC	1	868.765	4
101		13	max	0	1	.002	2	0	12	4.558e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.017	4	-8.691e-4	1	NC	1	1152.704	4
103		14	max	0	1	.002	2	0	12	4.558e-3	5	NC	1	NC	1
104			min	0	3	-.001	3	-.012	4	-8.691e-4	1	NC	1	1616.046	4
105		15	max	0	1	.002	2	0	12	4.558e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.008	4	-8.691e-4	1	NC	1	2452.186	4
107		16	max	0	1	.001	2	0	12	4.558e-3	5	NC	1	NC	1
108			min	0	3	0	3	-.005	4	-8.691e-4	1	NC	1	4210.92	4
109		17	max	0	1	0	2	0	12	4.558e-3	5	NC	1	NC	1
110			min	0	3	0	3	-.002	4	-8.691e-4	1	NC	1	9027.551	4
111		18	max	0	1	0	2	0	12	4.558e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-8.691e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	4.558e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-8.691e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.02	2	.004	1	2.134e-3	4	NC	3	NC	2
116			min	-.009	3	-.013	3	-.019	5	3.655e-6	10	1512.057	2	7788.748	1
117		2	max	.01	1	.019	2	.004	1	2.157e-3	4	NC	3	NC	2
118			min	-.008	3	-.013	3	-.019	5	2.936e-6	10	1612.272	2	8444.28	1
119		3	max	.009	1	.017	2	.003	1	2.18e-3	4	NC	3	NC	2
120			min	-.008	3	-.012	3	-.018	5	2.217e-6	10	1726.403	2	9222.607	1
121		4	max	.009	1	.016	2	.003	1	2.204e-3	4	NC	3	NC	1
122			min	-.007	3	-.011	3	-.017	5	1.499e-6	10	1857.222	2	NC	1
123		5	max	.008	1	.015	2	.003	1	2.227e-3	4	NC	3	NC	1
124			min	-.007	3	-.011	3	-.016	5	7.797e-7	10	2008.296	2	NC	1
125		6	max	.008	1	.014	2	.002	1	2.25e-3	4	NC	3	NC	1
126			min	-.006	3	-.01	3	-.015	5	6.092e-8	10	2184.293	2	NC	1
127		7	max	.007	1	.013	2	.002	1	2.274e-3	4	NC	3	NC	1
128			min	-.006	3	-.009	3	-.014	5	-6.579e-7	10	2391.44	2	NC	1
129		8	max	.006	1	.011	2	.002	1	2.297e-3	4	NC	3	NC	1
130			min	-.005	3	-.009	3	-.013	5	-1.377e-6	10	2638.23	2	NC	1
131		9	max	.006	1	.01	2	.002	1	2.32e-3	4	NC	3	NC	1
132			min	-.005	3	-.008	3	-.012	5	-3.484e-6	2	2936.554	2	NC	1
133		10	max	.005	1	.009	2	.001	1	2.344e-3	4	NC	3	NC	1
134			min	-.004	3	-.007	3	-.011	5	-7.649e-6	2	3303.589	2	NC	1
135		11	max	.005	1	.008	2	.001	1	2.367e-3	4	NC	3	NC	1
136			min	-.004	3	-.007	3	-.01	5	-1.181e-5	2	3765.102	2	NC	1
137		12	max	.004	1	.007	2	0	1	2.39e-3	4	NC	3	NC	1
138			min	-.003	3	-.006	3	-.009	5	-1.598e-5	2	4361.577	2	NC	1
139		13	max	.003	1	.006	2	0	1	2.414e-3	4	NC	3	NC	1
140			min	-.003	3	-.005	3	-.007	5	-2.015e-5	2	5160.487	2	NC	1
141		14	max	.003	1	.005	2	0	1	2.437e-3	4	NC	3	NC	1
142			min	-.002	3	-.004	3	-.006	5	-2.431e-5	2	6283.278	2	NC	1
143		15	max	.002	1	.004	2	0	1	2.461e-3	4	NC	3	NC	1
144			min	-.002	3	-.003	3	-.005	5	-2.848e-5	2	7972.823	2	NC	1
145		16	max	.002	1	.003	2	0	1	2.484e-3	4	NC	1	NC	1
146			min	-.001	3	-.003	3	-.004	5	-3.264e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	2.507e-3	4	NC	1	NC	1
148			min	0	3	-.002	3	-.003	5	-3.681e-5	2	NC	1	NC	1
149		18	max	0	1	0	2	0	1	2.531e-3	4	NC	1	NC	1
150			min	0	3	0	3	-.001	5	-4.097e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.554e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.514e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.025e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.163e-3	4	NC	1	NC	1
155		2	max	0	9	.001	2	.006	4	1.7e-5	2	NC	1	NC	1
156			min	0	2	-.001	3	0	2	-1.148e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	9	.003	2	.012	4	1.515e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-1.133e-3	4	NC	1	NC	1
159		4	max	0	9	.004	2	.019	4	1.474e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	2	-1.118e-3	4	NC	1	NC	1
161		5	max	0	9	.005	1	.025	4	1.433e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	2	-1.104e-3	4	8974.265	1	NC	1
163		6	max	0	9	.006	1	.031	4	1.393e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	-1.089e-3	4	7105.389	1	NC	1
165		7	max	0	9	.008	1	.037	4	1.59e-5	3	NC	3	NC	1
166			min	0	2	-.008	3	0	1	-1.074e-3	4	5833.491	1	NC	1
167		8	max	0	9	.009	1	.044	4	2.497e-5	3	NC	3	NC	1
168			min	0	2	-.01	3	0	1	-1.059e-3	4	4907.464	1	NC	1
169		9	max	0	9	.011	1	.05	4	3.403e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	-1.044e-3	4	4201.691	1	NC	1
171		10	max	0	9	.013	1	.056	4	4.31e-5	3	NC	3	NC	1
172			min	0	2	-.012	3	0	1	-1.03e-3	4	3646.141	1	NC	1
173		11	max	.001	9	.014	1	.062	4	5.216e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	-.001	1	-1.015e-3	4	3198.466	1	NC	1
175		12	max	.001	9	.016	1	.068	4	6.123e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-9.999e-4	4	2831.377	1	NC	1
177		13	max	.001	9	.018	1	.073	4	7.029e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.001	1	-9.851e-4	4	2526.357	1	NC	1
179		14	max	.001	9	.02	1	.079	4	7.936e-5	3	NC	3	NC	1
180			min	-.001	2	-.016	3	-.001	1	-9.704e-4	4	2270.306	1	NC	1
181		15	max	.002	9	.022	1	.085	4	8.843e-5	3	NC	3	NC	1
182			min	-.001	2	-.017	3	-.002	1	-9.556e-4	4	2053.639	1	NC	1
183		16	max	.002	9	.025	1	.09	4	9.749e-5	3	NC	3	NC	1
184			min	-.002	2	-.018	3	-.002	1	-9.408e-4	4	1869.145	1	NC	1
185		17	max	.002	9	.027	1	.096	4	1.066e-4	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-9.26e-4	4	1711.28	1	NC	1
187		18	max	.002	9	.029	1	.101	4	1.156e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	-.002	1	-9.112e-4	4	1575.708	1	NC	1
189		19	max	.002	9	.032	1	.106	4	1.247e-4	3	NC	3	NC	1
190			min	-.002	2	-.02	3	-.002	1	-8.964e-4	4	1458.992	1	NC	1
191	M8	1	max	.008	1	.023	2	.002	1	4.294e-3	4	NC	1	NC	2
192			min	-.002	3	-.015	3	-.112	4	-1.004e-4	3	NC	1	172.764	4
193		2	max	.008	1	.022	2	.002	1	4.294e-3	4	NC	1	NC	2
194			min	-.002	3	-.014	3	-.103	4	-1.004e-4	3	NC	1	188.34	4
195		3	max	.007	1	.021	2	.002	1	4.294e-3	4	NC	1	NC	1
196			min	-.002	3	-.013	3	-.093	4	-1.004e-4	3	NC	1	206.88	4
197		4	max	.007	1	.019	2	.002	1	4.294e-3	4	NC	1	NC	1
198			min	-.002	3	-.012	3	-.084	4	-1.004e-4	3	NC	1	229.165	4
199		5	max	.006	1	.018	2	.001	1	4.294e-3	4	NC	1	NC	1
200			min	-.001	3	-.012	3	-.075	4	-1.004e-4	3	NC	1	256.257	4
201		6	max	.006	1	.017	2	.001	1	4.294e-3	4	NC	1	NC	1
202			min	-.001	3	-.011	3	-.067	4	-1.004e-4	3	NC	1	289.637	4
203		7	max	.006	1	.015	2	.001	1	4.294e-3	4	NC	1	NC	1
204			min	-.001	3	-.01	3	-.058	4	-1.004e-4	3	NC	1	331.41	4
205		8	max	.005	1	.014	2	0	1	4.294e-3	4	NC	1	NC	1
206			min	-.001	3	-.009	3	-.05	4	-1.004e-4	3	NC	1	384.662	4
207		9	max	.005	1	.013	2	0	1	4.294e-3	4	NC	1	NC	1
208			min	-.001	3	-.008	3	-.043	4	-1.004e-4	3	NC	1	454.058	4
209		10	max	.004	1	.012	2	0	1	4.294e-3	4	NC	1	NC	1
210			min	0	3	-.007	3	-.035	4	-1.004e-4	3	NC	1	546.932	4
211		11	max	.004	1	.01	2	0	1	4.294e-3	4	NC	1	NC	1
212			min	0	3	-.007	3	-.029	4	-1.004e-4	3	NC	1	675.363	4
213		12	max	.003	1	.009	2	0	1	4.294e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	1	0	2	.009	4	1.689e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-1.042e-3	4	NC	1	5125.907	4
273		4	max	0	1	0	2	.014	4	6.154e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-1.147e-3	4	NC	1	3382.169	4
275		5	max	0	1	0	2	.018	4	-3.396e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	0	1	-1.253e-3	4	NC	1	2513.394	4
277		6	max	0	1	0	2	.023	4	-1.022e-5	12	NC	1	NC	1
278			min	0	10	-.004	3	-.002	1	-1.359e-3	4	NC	1	1994.17	4
279		7	max	0	1	.001	2	.028	4	-1.705e-5	12	NC	1	NC	1
280			min	0	10	-.004	3	-.002	1	-1.464e-3	4	NC	1	1649.387	4
281		8	max	0	1	.001	2	.033	5	-2.388e-5	12	NC	1	NC	1
282			min	0	10	-.005	3	-.003	1	-1.57e-3	4	NC	1	1400.45	5
283		9	max	0	1	.002	2	.038	5	-3.071e-5	12	NC	1	NC	1
284			min	0	10	-.005	3	-.004	1	-1.676e-3	4	NC	1	1214.609	5
285		10	max	0	1	.002	2	.043	5	-3.754e-5	12	NC	1	NC	2
286			min	0	10	-.006	3	-.005	1	-1.781e-3	4	NC	1	1070.739	5
287		11	max	0	1	.003	2	.048	5	-4.437e-5	12	NC	1	NC	2
288			min	0	10	-.006	3	-.006	1	-1.887e-3	4	NC	1	956.078	5
289		12	max	0	1	.003	2	.053	5	-4.868e-5	10	NC	1	NC	2
290			min	0	10	-.006	3	-.007	1	-1.993e-3	4	NC	1	862.517	5
291		13	max	0	1	.004	2	.059	5	-5.267e-5	10	NC	1	NC	2
292			min	0	10	-.007	3	-.008	1	-2.098e-3	4	NC	1	784.657	5
293		14	max	.001	1	.005	2	.064	5	-5.667e-5	10	NC	3	NC	2
294			min	0	10	-.007	3	-.009	1	-2.204e-3	4	9410.903	2	718.766	5
295		15	max	.001	1	.006	2	.07	5	-6.067e-5	10	NC	3	NC	2
296			min	0	10	-.007	3	-.01	1	-2.31e-3	4	8001.606	2	662.186	5
297		16	max	.001	1	.007	2	.075	5	-6.467e-5	10	NC	3	NC	2
298			min	0	10	-.007	3	-.011	1	-2.416e-3	4	6889.311	2	612.972	5
299		17	max	.001	1	.008	1	.081	5	-6.867e-5	10	NC	3	NC	2
300			min	0	10	-.007	3	-.012	1	-2.521e-3	4	5977.908	1	569.672	5
301		18	max	.001	1	.009	1	.087	5	-7.267e-5	10	NC	3	NC	2
302			min	0	10	-.007	3	-.013	1	-2.627e-3	4	5253.855	1	531.18	5
303		19	max	.001	1	.01	1	.093	5	-7.667e-5	10	NC	3	NC	3
304			min	0	10	-.007	3	-.013	1	-2.733e-3	4	4675.901	1	496.642	5
305	M12	1	max	.003	1	.007	2	.011	1	5.866e-3	4	NC	1	NC	3
306			min	0	3	-.005	3	-.102	5	6.793e-5	10	NC	1	189.221	5
307		2	max	.003	1	.007	2	.01	1	5.866e-3	4	NC	1	NC	3
308			min	0	3	-.005	3	-.094	5	6.793e-5	10	NC	1	206.277	5
309		3	max	.002	1	.007	2	.009	1	5.866e-3	4	NC	1	NC	3
310			min	0	3	-.005	3	-.085	5	6.793e-5	10	NC	1	226.579	5
311		4	max	.002	1	.006	2	.008	1	5.866e-3	4	NC	1	NC	3
312			min	0	3	-.004	3	-.077	5	6.793e-5	10	NC	1	250.981	5
313		5	max	.002	1	.006	2	.007	1	5.866e-3	4	NC	1	NC	3
314			min	0	3	-.004	3	-.069	5	6.793e-5	10	NC	1	280.648	5
315		6	max	.002	1	.005	2	.007	1	5.866e-3	4	NC	1	NC	3
316			min	0	3	-.004	3	-.061	5	6.793e-5	10	NC	1	317.199	5
317		7	max	.002	1	.005	2	.006	1	5.866e-3	4	NC	1	NC	3
318			min	0	3	-.004	3	-.053	5	6.793e-5	10	NC	1	362.94	5
319		8	max	.002	1	.004	2	.005	1	5.866e-3	4	NC	1	NC	2
320			min	0	3	-.003	3	-.046	5	6.793e-5	10	NC	1	421.25	5
321		9	max	.002	1	.004	2	.004	1	5.866e-3	4	NC	1	NC	2
322			min	0	3	-.003	3	-.039	5	6.793e-5	10	NC	1	497.236	5
323		10	max	.001	1	.004	2	.003	1	5.866e-3	4	NC	1	NC	2
324			min	0	3	-.003	3	-.032	5	6.793e-5	10	NC	1	598.928	5
325		11	max	.001	1	.003	2	.003	1	5.866e-3	4	NC	1	NC	2
326			min	0	3	-.002	3	-.026	5	6.793e-5	10	NC	1	739.552	5
327		12	max	.001	1	.003	2	.002	1	5.866e-3	4	NC	1	NC	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.021	5	6.793e-5	10	NC	1	942.179	5
329		13	max	0	1	.002	2	.002	1	5.866e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.015	5	6.793e-5	10	NC	1	1250.081	5
331		14	max	0	1	.002	2	.001	1	5.866e-3	4	NC	1	NC	1
332			min	0	3	-.001	3	-.011	5	6.793e-5	10	NC	1	1752.521	5
333		15	max	0	1	.002	2	0	1	5.866e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.007	5	6.793e-5	10	NC	1	2659.206	5
335		16	max	0	1	.001	2	0	1	5.866e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.004	5	6.793e-5	10	NC	1	4566.297	5
337		17	max	0	1	0	2	0	1	5.866e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.002	5	6.793e-5	10	NC	1	9789.155	5
339		18	max	0	1	0	2	0	1	5.866e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	6.793e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	5.866e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	6.793e-5	10	NC	1	NC	1
343	M1	1	max	.005	3	.022	3	.01	5	2.007e-2	1	NC	1	NC	1
344			min	-.006	2	-.031	1	-.004	1	-1.612e-2	3	NC	1	NC	1
345		2	max	.005	3	.012	3	.015	5	9.657e-3	1	NC	4	NC	2
346			min	-.006	2	-.017	1	-.01	1	-7.977e-3	3	3136.687	1	8863.661	1
347		3	max	.005	3	.002	3	.02	5	4.917e-4	5	NC	5	NC	2
348			min	-.006	2	-.003	1	-.013	1	-5.592e-4	1	1622.445	1	5056.738	5
349		4	max	.005	3	.009	1	.025	5	4.877e-4	5	NC	5	NC	2
350			min	-.006	2	-.006	3	-.015	1	-4.597e-4	1	1148.091	1	3212.898	5
351		5	max	.005	3	.019	1	.031	5	4.836e-4	5	NC	5	NC	2
352			min	-.006	2	-.012	3	-.015	1	-3.603e-4	1	920.188	1	2311.084	5
353		6	max	.005	3	.027	1	.037	5	4.795e-4	5	NC	5	NC	2
354			min	-.006	2	-.017	3	-.014	1	-2.609e-4	1	791.466	1	1782.797	5
355		7	max	.005	3	.033	1	.043	5	4.754e-4	5	NC	5	NC	2
356			min	-.006	2	-.021	3	-.013	1	-1.614e-4	1	713.631	1	1439.455	5
357		8	max	.005	3	.038	1	.049	5	4.713e-4	5	NC	5	NC	2
358			min	-.006	2	-.024	3	-.01	1	-6.198e-5	1	666.607	1	1200.619	5
359		9	max	.005	3	.041	1	.056	5	4.673e-4	5	NC	5	NC	1
360			min	-.006	2	-.025	3	-.007	1	3.633e-6	2	641.026	1	1018.272	4
361		10	max	.005	3	.041	1	.063	5	4.828e-4	4	NC	5	NC	1
362			min	-.006	2	-.025	3	-.004	1	1.38e-5	10	632.578	1	876.195	4
363		11	max	.005	3	.04	1	.07	4	5.e-4	4	NC	5	NC	1
364			min	-.006	2	-.024	3	-.001	1	2.135e-5	10	639.965	1	768.303	4
365		12	max	.005	3	.038	1	.077	4	5.171e-4	4	NC	5	NC	2
366			min	-.006	2	-.022	3	0	10	2.64e-5	12	664.369	1	684.558	4
367		13	max	.005	3	.033	1	.085	4	5.343e-4	4	NC	5	NC	2
368			min	-.006	2	-.019	3	0	12	2.811e-5	12	709.942	1	618.456	4
369		14	max	.005	3	.026	1	.092	4	5.515e-4	4	NC	5	NC	2
370			min	-.006	2	-.015	3	0	12	2.983e-5	12	785.773	1	565.634	4
371		15	max	.005	3	.018	1	.099	4	6.341e-4	1	NC	5	NC	2
372			min	-.007	2	-.011	3	0	12	3.155e-5	12	911.34	1	523.074	4
373		16	max	.005	3	.008	1	.105	4	9.196e-4	4	NC	5	NC	2
374			min	-.007	2	-.005	3	0	12	3.26e-5	12	1133.077	1	488.641	4
375		17	max	.005	3	.002	3	.111	4	9.215e-3	4	NC	5	NC	2
376			min	-.007	2	-.004	1	0	12	1.719e-5	10	1590.459	1	460.826	4
377		18	max	.005	3	.009	3	.116	4	1.124e-2	1	NC	4	NC	2
378			min	-.007	2	-.018	1	0	10	-3.802e-3	3	3065.63	1	438.47	4
379		19	max	.005	3	.017	3	.12	4	2.254e-2	1	NC	1	NC	1
380			min	-.007	2	-.034	1	-.003	1	-7.704e-3	3	NC	1	421.228	4
381	M5	1	max	.014	3	.066	3	.01	5	4.023e-6	4	NC	1	NC	1
382			min	-.02	2	-.094	1	-.005	1	5.36e-8	10	NC	1	NC	1
383		2	max	.014	3	.036	3	.014	5	2.324e-4	5	NC	5	NC	1
384			min	-.02	2	-.05	1	-.004	1	-9.344e-5	1	1059.765	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.014	3	.008	3	.02	5	4.57e-4	5	NC	5	NC	1
386			min	-.02	2	-.009	1	-.004	1	-1.853e-4	1	545.755	1	NC	1
387		4	max	.014	3	.026	1	.025	5	4.746e-4	5	NC	5	NC	1
388			min	-.02	2	-.015	3	-.003	1	-1.73e-4	1	385.397	1	NC	1
389		5	max	.014	3	.056	1	.031	5	4.921e-4	5	NC	15	NC	1
390			min	-.02	2	-.034	3	-.003	1	-1.607e-4	1	308.339	1	NC	1
391		6	max	.014	3	.081	1	.038	5	5.097e-4	5	NC	15	NC	1
392			min	-.02	2	-.049	3	-.003	1	-1.484e-4	1	264.767	1	NC	1
393		7	max	.014	3	.1	1	.045	5	5.272e-4	5	NC	15	NC	1
394			min	-.02	2	-.06	3	-.002	1	-1.361e-4	1	238.358	1	NC	1
395		8	max	.014	3	.113	1	.052	5	5.448e-4	5	NC	15	NC	1
396			min	-.02	2	-.068	3	-.002	1	-1.237e-4	1	222.324	1	NC	1
397		9	max	.014	3	.122	1	.059	5	5.623e-4	5	9703.887	15	NC	1
398			min	-.02	2	-.072	3	-.002	1	-1.114e-4	1	213.496	1	NC	1
399		10	max	.014	3	.124	1	.066	5	5.799e-4	5	9602.735	15	NC	1
400			min	-.02	2	-.072	3	-.002	1	-9.911e-5	1	210.41	1	NC	1
401		11	max	.014	3	.122	1	.073	4	5.974e-4	5	9740.717	15	NC	1
402			min	-.02	2	-.07	3	-.002	1	-8.68e-5	1	212.614	1	NC	1
403		12	max	.014	3	.113	1	.081	4	6.149e-4	5	NC	15	NC	1
404			min	-.02	2	-.064	3	-.002	1	-7.449e-5	1	220.487	1	NC	1
405		13	max	.014	3	.099	1	.088	4	6.325e-4	5	NC	15	NC	1
406			min	-.021	2	-.055	3	-.002	1	-6.217e-5	1	235.402	1	NC	1
407		14	max	.014	3	.08	1	.094	4	6.5e-4	5	NC	15	NC	1
408			min	-.021	2	-.044	3	-.002	1	-4.986e-5	1	260.38	1	9402.121	4
409		15	max	.014	3	.054	1	.101	4	6.676e-4	5	NC	15	NC	1
410			min	-.021	2	-.03	3	-.002	1	-3.755e-5	1	301.919	1	9256.147	4
411		16	max	.014	3	.023	1	.106	4	1.013e-3	5	NC	5	NC	1
412			min	-.021	2	-.013	3	-.002	1	-3.377e-5	2	375.599	1	9985.022	4
413		17	max	.014	3	.005	3	.112	4	9.254e-3	4	NC	5	NC	1
414			min	-.021	2	-.013	1	-.002	1	-1.868e-4	1	528.915	1	NC	1
415		18	max	.014	3	.026	3	.117	4	4.748e-3	4	NC	5	NC	1
416			min	-.021	2	-.056	1	-.002	1	-9.577e-5	1	1024.479	1	NC	1
417		19	max	.014	3	.048	3	.12	4	1.503e-6	5	NC	1	NC	1
418			min	-.021	2	-.103	1	-.002	1	-8.372e-8	3	NC	1	NC	1
419	M9	1	max	.005	3	.022	3	.008	5	1.612e-2	3	NC	1	NC	1
420			min	-.006	2	-.031	1	-.006	1	-2.007e-2	1	NC	1	NC	1
421		2	max	.005	3	.012	3	.008	5	7.997e-3	3	NC	4	NC	1
422			min	-.006	2	-.017	1	-.001	1	-9.916e-3	1	3137.535	1	NC	1
423		3	max	.005	3	.002	3	.008	4	4.547e-5	1	NC	5	NC	2
424			min	-.006	2	-.003	1	0	3	9.014e-6	10	1622.897	1	6433.157	1
425		4	max	.005	3	.009	1	.01	4	1.231e-5	5	NC	5	NC	2
426			min	-.006	2	-.006	3	0	3	-3.712e-5	1	1148.412	1	5457.094	1
427		5	max	.005	3	.019	1	.013	4	2.113e-6	3	NC	5	NC	2
428			min	-.006	2	-.012	3	0	3	-1.197e-4	1	920.437	1	5415.619	1
429		6	max	.005	3	.027	1	.017	4	-4.499e-6	12	NC	5	NC	2
430			min	-.006	2	-.017	3	-.001	3	-2.023e-4	1	791.671	1	4387.832	4
431		7	max	.005	3	.033	1	.022	4	-1.011e-5	12	NC	5	NC	2
432			min	-.006	2	-.021	3	-.001	3	-2.849e-4	1	713.805	1	3018.463	4
433		8	max	.005	3	.038	1	.028	4	-1.572e-5	12	NC	5	NC	1
434			min	-.006	2	-.024	3	-.002	3	-3.675e-4	1	666.759	1	2216.336	4
435		9	max	.005	3	.041	1	.034	4	-2.133e-5	12	NC	5	NC	1
436			min	-.006	2	-.025	3	-.003	1	-4.501e-4	1	641.163	1	1706.314	4
437		10	max	.005	3	.041	1	.041	5	-2.695e-5	12	NC	5	NC	1
438			min	-.006	2	-.025	3	-.006	1	-5.327e-4	1	632.702	1	1361.737	4
439		11	max	.005	3	.04	1	.049	5	-3.256e-5	12	NC	5	NC	2
440			min	-.006	2	-.024	3	-.009	1	-6.153e-4	1	640.081	1	1117.848	4
441		12	max	.005	3	.038	1	.057	5	-3.817e-5	12	NC	5	NC	2





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**Anchor Designer™**  
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

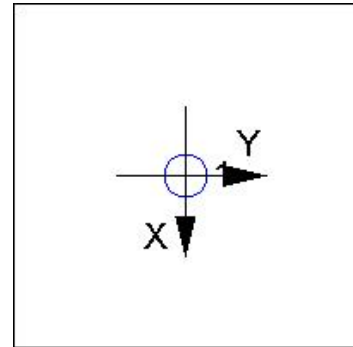
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Engineer:	HCV	Page:	5/5
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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

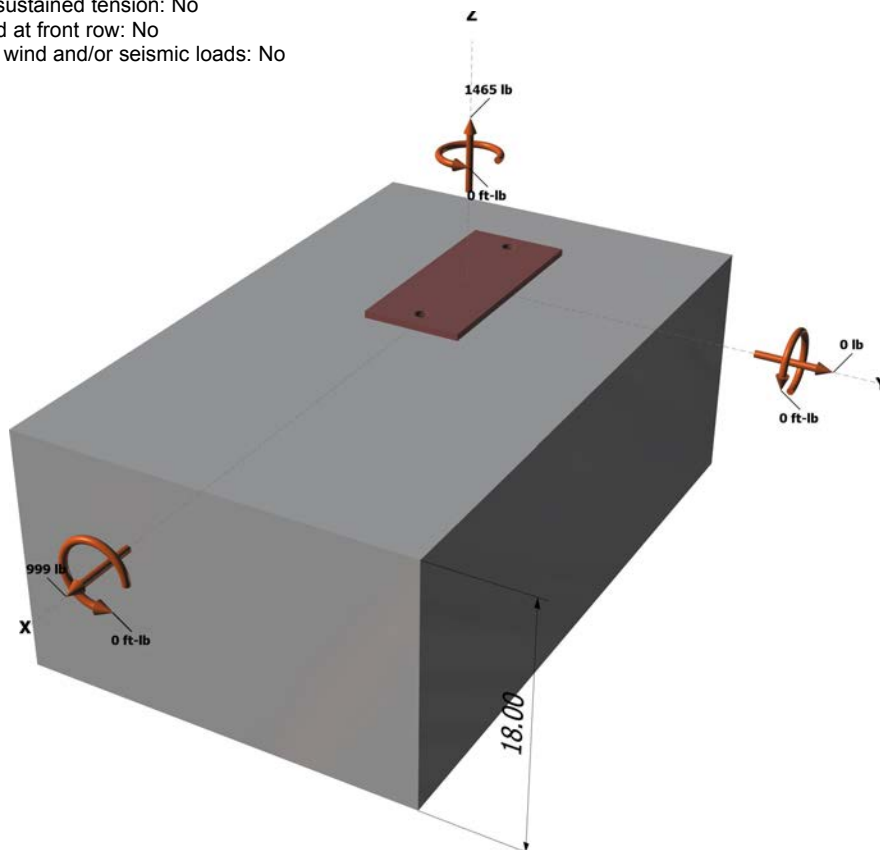
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

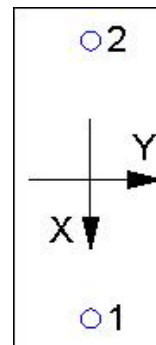
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$  (Eq. D-7)

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$  (Sec. D.4.1 & Eq. D-5)

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$  (Eq. D-16f)

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$  (Sec. D.4.1 & Eq. D-16b)

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.