

Û&@^α ÉQ&É		HÍ »Áq₃ ÐÂ^ā{ æÖ^•ă }
PÔX	Ùcə āāāÁÚXT æÄæ ā * Â^•c{	
	Ü^ ^•^} cæ^ Âæ& æā } •ÆÖÜÔÔÃ Æ€	

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	75 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	-1.413 k-ft
M_z =	-0.005 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	51%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	88.90 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.35 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-2.508 k-ft
M_z =	0.000 k-ft
P_n =	-0.947 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	74%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.773 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.009 k-ft
M_z =	0.000 k-ft
P_n =	2.755 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	38%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.009 k-ft
M_z =	0.000 k-ft
P_n =	2.814 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<u>33%</u>



DETAIL VIEW

5. FOUNDATION DESIGN CALCULATIONS

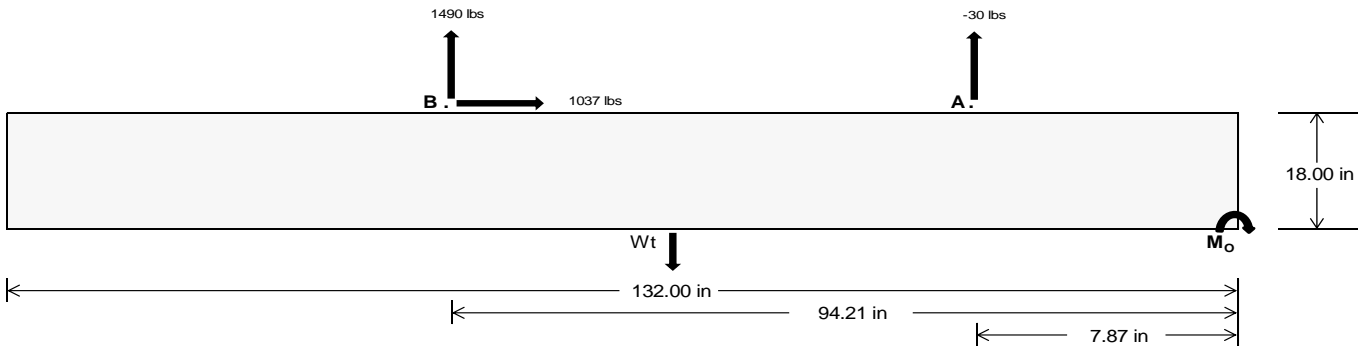
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>63.16</u>	<u>6468.09</u> k
Compressive Load =		<u>2304.77</u>	<u>4599.51</u> k
Lateral Load =		<u>5.64</u>	<u>4491.43</u> k
Moment (Weak Axis) =		<u>0.01</u>	<u>0.00</u> k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 158831.8$ in-lbs
Resisting Force Required = 2406.54 lbs
S.F. = 1.67
Weight Required = 4010.90 lbs
Minimum Width = 31 in
Weight Provided = 6180.63 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 31in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 1036.50 lbs
Friction = 0.4
Weight Required = 2591.26 lbs
Resisting Weight = 6180.63 lbs
Additional Weight Required = 0 lbs

Use a 132in long x 31in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 1036.50 lbs
Cohesion = 130 psf
Area = 28.42 ft²
Resisting = 3090.31 lbs
Additional Weight Required = 0 lbs

Use a 132in long x 31in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width
31 in 32 in 33 in 34 in
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.58 \text{ ft}) =$ 6181 lbs 6380 lbs 6579 lbs 6779 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in
F_A	681 lbs	681 lbs	681 lbs	681 lbs	960 lbs	960 lbs	960 lbs	960 lbs	1144 lbs	1144 lbs	1144 lbs	1144 lbs	59 lbs	59 lbs	59 lbs	59 lbs
F_B	591 lbs	591 lbs	591 lbs	591 lbs	2032 lbs	2032 lbs	2032 lbs	2032 lbs	1892 lbs	1892 lbs	1892 lbs	1892 lbs	-2981 lbs	-2981 lbs	-2981 lbs	-2981 lbs
F_V	88 lbs	88 lbs	88 lbs	88 lbs	1867 lbs	1867 lbs	1867 lbs	1867 lbs	1456 lbs	1456 lbs	1456 lbs	1456 lbs	-2073 lbs	-2073 lbs	-2073 lbs	-2073 lbs
P_{total}	7453 lbs	7652 lbs	7851 lbs	8051 lbs	9173 lbs	9373 lbs	9572 lbs	9771 lbs	9217 lbs	9416 lbs	9615 lbs	9815 lbs	787 lbs	906 lbs	1026 lbs	1146 lbs
M	2038 lbs-ft	2038 lbs-ft	2038 lbs-ft	2038 lbs-ft	2674 lbs-ft	2674 lbs-ft	2674 lbs-ft	2674 lbs-ft	3278 lbs-ft	3278 lbs-ft	3278 lbs-ft	3278 lbs-ft	4184 lbs-ft	4184 lbs-ft	4184 lbs-ft	4184 lbs-ft
e	0.27 ft	0.27 ft	0.26 ft	0.25 ft	0.29 ft	0.29 ft	0.28 ft	0.27 ft	0.36 ft	0.35 ft	0.34 ft	0.33 ft	5.32 ft	4.62 ft	4.08 ft	3.65 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	223.1 psf	223.0 psf	222.8 psf	222.6 psf	271.5 psf	269.8 psf	268.2 psf	266.7 psf	261.4 psf	260.0 psf	258.8 psf	257.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	301.4 psf	298.8 psf	296.3 psf	294.0 psf	374.1 psf	369.2 psf	364.7 psf	360.3 psf	387.3 psf	381.9 psf	377.0 psf	372.3 psf	1116.2 psf	256.4 psf	174.9 psf	145.9 psf

Maximum Bearing Pressure = 1116 psf
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 31in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

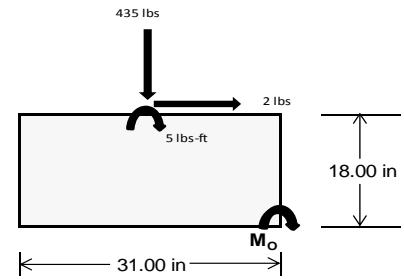
Overturning Check

$M_o = 553.7 \text{ ft-lbs}$
 Resisting Force Required = 428.66 lbs
 S.F. = 1.67
 Weight Required = 714.43 lbs
 Minimum Width = 31 in
 Weight Provided = 6180.63 lbs

A minimum 132in long x 31in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	31 in			31 in			31 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	181 lbs	402 lbs	181 lbs	435 lbs	1093 lbs	435 lbs	53 lbs	118 lbs	53 lbs
F_v	1 lbs	0 lbs	1 lbs	2 lbs	0 lbs	2 lbs	0 lbs	0 lbs	0 lbs
P_{total}	7833 lbs	6181 lbs	7833 lbs	7719 lbs	6181 lbs	7719 lbs	2290 lbs	6181 lbs	2290 lbs
M	3 lbs-ft	0 lbs-ft	3 lbs-ft	9 lbs-ft	0 lbs-ft	9 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft
f_{min}	275.4 psf	217.5 psf	275.4 psf	270.9 psf	217.5 psf	270.9 psf	80.6 psf	217.5 psf	80.6 psf
f_{max}	275.9 psf	217.5 psf	275.9 psf	272.4 psf	217.5 psf	272.4 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 276 psf
 Allowable Bearing Pressure = 1500 psf

Use a 132in long x 31in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 132in long x 31in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	1.027 k
Allowable Uplift =	1.214 k
Utilization =	<u>85%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.308 k
Allowable Uplift =	4.357 k
Utilization =	<u>53%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.773 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>24%</u>

Rear Strut

Maximum Axial Load =	4.276 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>58%</u>

Diagonal Strut

Maximum Axial Load =	2.814 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	53.78 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.076 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 75 \text{ in}$$

$$J = 0.432$$

$$207.485$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75$$

$$J = 0.432$$

$$131.948$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(lyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(lyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-134.509	-134.509	0	0
2	M14	y	-134.509	-134.509	0	0
3	M15	y	-224.182	-224.182	0	0
4	M16	y	-224.182	-224.182	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	302.645	302.645	0	0
2	M14	y	235.391	235.391	0	0
3	M15	y	134.509	134.509	0	0
4	M16	y	134.509	134.509	0	0

Load Combinations

	Description	S... P...	S... B...	Fa... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes Y		1 1.2	3 1.6	4 .5												
2	LRFD 1.2D + 1.0W + 0.5S	Yes Y		1 1.2	3 .5	4 1												
3	LRFD 0.9D + 1.0W	Yes Y		2 .9				5 1										
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2		6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56			6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2		6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56			6 1.25											



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 35° 160mph 30psf 6.25ft 7-10 NS.r3d] Page 19



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	37.705	1	187.834	2	3.383	3	.011	2	-.003	15	.653	3
28			min	2.213	15	-329.659	3	-20.899	1	0	15	-.061	1	-.299	2
29		15	max	37.705	1	78.31	2	5.517	3	.011	2	-.003	12	.81	3
30			min	2.213	15	-122.537	3	-1.25	10	0	15	-.067	1	-.391	2
31		16	max	37.705	1	84.584	3	30.56	1	.011	2	0	3	.823	3
32			min	2.213	15	-31.214	2	1.796	15	0	15	-.055	1	-.407	2
33		17	max	37.705	1	291.706	3	56.29	1	.011	2	.006	3	.693	3
34			min	2.213	15	-140.739	2	3.219	15	0	15	-.025	1	-.348	2
35		18	max	37.705	1	498.828	3	82.02	1	.011	2	.023	1	.418	3
36			min	2.213	15	-250.263	2	4.642	15	0	15	0	10	-.212	2
37		19	max	37.705	1	705.95	3	107.749	1	.011	2	.089	1	0	2
38			min	2.213	15	-359.787	2	6.065	15	0	15	.005	15	0	3
39	M14	1	max	23.771	1	432.49	2	-6.337	15	.011	3	.109	1	0	2
40			min	1.378	15	-594.562	3	-112.547	1	-.012	2	.006	15	0	3
41		2	max	23.771	1	322.966	2	-4.914	15	.011	3	.04	1	.357	3
42			min	1.378	15	-434.144	3	-86.817	1	-.012	2	.001	10	-.262	2
43		3	max	23.771	1	213.442	2	-3.491	15	.011	3	.008	3	.603	3
44			min	1.378	15	-273.726	3	-61.087	1	-.012	2	-.011	1	-.449	2
45		4	max	23.771	1	103.918	2	-2.068	15	.011	3	.002	3	.737	3
46			min	1.378	15	-113.309	3	-35.357	1	-.012	2	-.045	1	-.559	2
47		5	max	23.771	1	47.109	3	.644	10	.011	3	-.002	12	.76	3
48			min	1.378	15	-7.337	1	-9.628	1	-.012	2	-.061	1	-.593	2
49		6	max	23.771	1	207.526	3	16.102	1	.011	3	-.003	15	.672	3
50			min	1.378	15	-115.131	2	-3.929	3	-.012	2	-.058	1	-.551	2
51		7	max	23.771	1	367.944	3	41.832	1	.011	3	-.002	15	.472	3
52			min	1.378	15	-224.655	2	-1.794	3	-.012	2	-.038	1	-.433	2
53		8	max	23.771	1	528.362	3	67.562	1	.011	3	.005	2	.161	3
54			min	1.378	15	-334.179	2	.34	3	-.012	2	-.009	3	-.239	2
55		9	max	23.771	1	688.779	3	93.291	1	.011	3	.056	1	.041	1
56			min	1.378	15	-443.703	2	1.985	12	-.012	2	-.008	3	-.262	3
57		10	max	23.771	1	849.197	3	119.021	1	.011	3	.129	1	.377	2
58			min	1.378	15	-553.228	2	3.408	12	-.012	2	-.006	3	-.796	3
59		11	max	23.771	1	443.703	2	-1.985	12	.012	2	.056	1	.041	1
60			min	1.378	15	-688.779	3	-93.291	1	-.011	3	-.008	3	-.262	3
61		12	max	23.771	1	334.179	2	-.34	3	.012	2	.005	2	.161	3
62			min	1.378	15	-528.362	3	-67.562	1	-.011	3	-.009	3	-.239	2
63		13	max	23.771	1	224.655	2	1.794	3	.012	2	-.002	15	.472	3
64			min	1.378	15	-367.944	3	-41.832	1	-.011	3	-.038	1	-.433	2
65		14	max	23.771	1	115.131	2	3.929	3	.012	2	-.003	15	.672	3
66			min	1.378	15	-207.526	3	-16.102	1	-.011	3	-.058	1	-.551	2
67		15	max	23.771	1	7.337	1	9.628	1	.012	2	-.002	12	.76	3
68			min	1.378	15	-47.109	3	-.644	10	-.011	3	-.061	1	-.593	2
69		16	max	23.771	1	113.309	3	35.357	1	.012	2	.002	3	.737	3
70			min	1.378	15	-103.918	2	2.068	15	-.011	3	-.045	1	-.559	2
71		17	max	23.771	1	273.726	3	61.087	1	.012	2	.008	3	.603	3
72			min	1.378	15	-213.442	2	3.491	15	-.011	3	-.011	1	-.449	2
73		18	max	23.771	1	434.144	3	86.817	1	.012	2	.04	1	.357	3
74			min	1.378	15	-322.966	2	4.914	15	-.011	3	.001	10	-.262	2
75		19	max	23.771	1	594.562	3	112.547	1	.012	2	.109	1	0	2
76			min	1.378	15	-432.49	2	6.337	15	-.011	3	.006	15	0	3
77	M15	1	max	-1.427	15	642.782	2	-6.335	15	.013	2	.109	1	0	2
78			min	-24.226	1	-358.52	3	-112.618	1	-.01	3	.006	15	0	3
79		2	max	-1.427	15	470.985	2	-4.911	15	.013	2	.04	1	.218	3
80			min	-24.226	1	-268.159	3	-86.888	1	-.01	3	.001	10	-.387	2
81		3	max	-1.427	15	299.188	2	-3.488	15	.013	2	.007	3	.372	3
82			min	-24.226	1	-177.799	3	-61.158	1	-.01	3	-.011	1	-.654	2
83		4	max	-1.427	15	127.391	2	-2.065	15	.013	2	.001	3	.465	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-24.226	1	-87.438	3	-35.428	1	-.01	3	-.045	1	-.802	2
85		5	max	-1.427	15	2.923	3	.512	10	.013	2	-.002	12	.494	3
86			min	-24.226	1	-44.406	2	-9.699	1	-.01	3	-.061	1	-.831	2
87		6	max	-1.427	15	93.283	3	16.031	1	.013	2	-.003	15	.46	3
88			min	-24.226	1	-216.203	2	-3.503	3	-.01	3	-.058	1	-.741	2
89		7	max	-1.427	15	183.644	3	41.761	1	.013	2	-.002	15	.364	3
90			min	-24.226	1	-388.001	2	-1.369	3	-.01	3	-.038	1	-.531	2
91		8	max	-1.427	15	274.005	3	67.491	1	.013	2	.005	2	.205	3
92			min	-24.226	1	-559.798	2	.766	3	-.01	3	-.009	3	-.202	2
93		9	max	-1.427	15	364.365	3	93.22	1	.013	2	.055	1	.247	2
94			min	-24.226	1	-731.595	2	2.241	12	-.01	3	-.007	3	-.016	3
95		10	max	-1.427	15	599.761	1	77.624	9	.013	2	.129	1	.814	2
96			min	-24.226	1	-903.392	2	-118.95	1	-.01	3	-.005	3	-.301	3
97		11	max	-1.427	15	731.595	2	-2.241	12	.01	3	.055	1	.247	2
98			min	-24.226	1	-364.365	3	-93.22	1	-.013	2	-.007	3	-.016	3
99		12	max	-1.427	15	559.798	2	-.766	3	.01	3	.005	2	.205	3
100			min	-24.226	1	-274.005	3	-67.491	1	-.013	2	-.009	3	-.202	2
101		13	max	-1.427	15	388.001	2	1.369	3	.01	3	-.002	15	.364	3
102			min	-24.226	1	-183.644	3	-41.761	1	-.013	2	-.038	1	-.531	2
103		14	max	-1.427	15	216.203	2	3.503	3	.01	3	-.003	15	.46	3
104			min	-24.226	1	-93.283	3	-16.031	1	-.013	2	-.058	1	-.741	2
105		15	max	-1.427	15	44.406	2	9.699	1	.01	3	-.002	12	.494	3
106			min	-24.226	1	-2.923	3	-.512	10	-.013	2	-.061	1	-.831	2
107		16	max	-1.427	15	87.438	3	35.428	1	.01	3	.001	3	.465	3
108			min	-24.226	1	-127.391	2	2.065	15	-.013	2	-.045	1	-.802	2
109		17	max	-1.427	15	177.799	3	61.158	1	.01	3	.007	3	.372	3
110			min	-24.226	1	-299.188	2	3.488	15	-.013	2	-.011	1	-.654	2
111		18	max	-1.427	15	268.159	3	86.888	1	.01	3	.04	1	.218	3
112			min	-24.226	1	-470.985	2	4.911	15	-.013	2	.001	10	-.387	2
113		19	max	-1.427	15	358.52	3	112.618	1	.01	3	.109	1	0	2
114			min	-24.226	1	-642.782	2	6.335	15	-.013	2	.006	15	0	3
115	M16	1	max	-2.405	15	574.176	2	-6.072	15	.005	2	.091	1	0	2
116			min	-41.347	1	-294.176	3	-108.255	1	-.01	3	.005	15	0	3
117		2	max	-2.405	15	402.379	2	-4.649	15	.005	2	.025	1	.173	3
118			min	-41.347	1	-203.816	3	-82.525	1	-.01	3	0	10	-.339	2
119		3	max	-2.405	15	230.582	2	-3.226	15	.005	2	.004	3	.283	3
120			min	-41.347	1	-113.455	3	-56.795	1	-.01	3	-.024	1	-.559	2
121		4	max	-2.405	15	58.784	2	-1.803	15	.005	2	-.001	12	.33	3
122			min	-41.347	1	-23.094	3	-31.065	1	-.01	3	-.054	1	-.659	2
123		5	max	-2.405	15	67.266	3	.779	10	.005	2	-.003	12	.315	3
124			min	-41.347	1	-113.013	2	-5.336	1	-.01	3	-.067	1	-.64	2
125		6	max	-2.405	15	157.627	3	20.394	1	.005	2	-.003	15	.237	3
126			min	-41.347	1	-284.81	2	-1.997	3	-.01	3	-.062	1	-.502	2
127		7	max	-2.405	15	247.988	3	46.124	1	.005	2	-.002	15	.096	3
128			min	-41.347	1	-456.607	2	.137	3	-.01	3	-.038	1	-.245	2
129		8	max	-2.405	15	338.349	3	71.854	1	.005	2	.006	2	.132	2
130			min	-41.347	1	-628.404	2	1.749	12	-.01	3	-.007	3	-.107	3
131		9	max	-2.405	15	428.709	3	97.583	1	.005	2	.061	1	.628	2
132			min	-41.347	1	-800.201	2	3.172	12	-.01	3	-.005	3	-.374	3
133		10	max	-2.405	15	971.999	2	80.77	9	.005	2	.138	1	1.243	2
134			min	-41.347	1	-560.273	10	-123.313	1	-.01	3	0	3	-.703	3
135		11	max	-2.405	15	800.201	2	-3.172	12	.01	3	.061	1	.628	2
136			min	-41.347	1	-428.709	3	-97.583	1	-.005	2	-.005	3	-.374	3
137		12	max	-2.405	15	628.404	2	-1.749	12	.01	3	.006	2	.132	2
138			min	-41.347	1	-338.349	3	-71.854	1	-.005	2	-.007	3	-.107	3
139		13	max	-2.405	15	456.607	2	-.137	3	.01	3	-.002	15	.096	3
140			min	-41.347	1	-247.988	3	-46.124	1	-.005	2	-.038	1	-.245	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-2.405	15	284.81	2	1.997	3	.01	3	-.003	15	.237	3
142			min	-41.347	1	-157.627	3	-20.394	1	-.005	2	-.062	1	-.502	2
143		15	max	-2.405	15	113.013	2	5.336	1	.01	3	-.003	12	.315	3
144			min	-41.347	1	-67.266	3	-.779	10	-.005	2	-.067	1	-.64	2
145		16	max	-2.405	15	23.094	3	31.065	1	.01	3	-.001	12	.33	3
146			min	-41.347	1	-58.784	2	1.803	15	-.005	2	-.054	1	-.659	2
147		17	max	-2.405	15	113.455	3	56.795	1	.01	3	.004	3	.283	3
148			min	-41.347	1	-230.582	2	3.226	15	-.005	2	-.024	1	-.559	2
149		18	max	-2.405	15	203.816	3	82.525	1	.01	3	.025	1	.173	3
150			min	-41.347	1	-402.379	2	4.649	15	-.005	2	0	10	-.339	2
151		19	max	-2.405	15	294.176	3	108.255	1	.01	3	.091	1	0	2
152			min	-41.347	1	-574.176	2	6.072	15	-.005	2	.005	15	0	3
153	M2	1	max	940.173	2	2.02	4	.126	1	0	2	0	3	0	1
154			min	-1391.054	3	.475	15	.007	15	0	1	0	2	0	1
155		2	max	940.694	2	1.901	4	.126	1	0	2	0	1	0	15
156			min	-1390.664	3	.447	15	.007	15	0	1	0	10	0	4
157		3	max	941.215	2	1.782	4	.126	1	0	2	0	1	0	15
158			min	-1390.273	3	.419	15	.007	15	0	1	0	10	-.001	4
159		4	max	941.735	2	1.664	4	.126	1	0	2	0	1	0	15
160			min	-1389.883	3	.391	15	.007	15	0	1	0	15	-.002	4
161		5	max	942.256	2	1.545	4	.126	1	0	2	0	1	0	15
162			min	-1389.492	3	.363	15	.007	15	0	1	0	15	-.003	4
163		6	max	942.777	2	1.426	4	.126	1	0	2	0	1	0	15
164			min	-1389.102	3	.335	15	.007	15	0	1	0	15	-.003	4
165		7	max	943.298	2	1.307	4	.126	1	0	2	0	1	0	15
166			min	-1388.711	3	.307	15	.007	15	0	1	0	15	-.004	4
167		8	max	943.818	2	1.188	4	.126	1	0	2	0	1	0	15
168			min	-1388.321	3	.279	15	.007	15	0	1	0	15	-.004	4
169		9	max	944.339	2	1.069	4	.126	1	0	2	0	1	-.001	15
170			min	-1387.93	3	.248	12	.007	15	0	1	0	15	-.004	4
171		10	max	944.86	2	.95	4	.126	1	0	2	0	1	-.001	15
172			min	-1387.54	3	.202	12	.007	15	0	1	0	15	-.005	4
173		11	max	945.38	2	.854	2	.126	1	0	2	0	1	-.001	15
174			min	-1387.149	3	.156	12	.007	15	0	1	0	15	-.005	4
175		12	max	945.901	2	.762	2	.126	1	0	2	0	1	-.001	15
176			min	-1386.758	3	.109	12	.007	15	0	1	0	15	-.005	4
177		13	max	946.422	2	.669	2	.126	1	0	2	0	1	-.001	15
178			min	-1386.368	3	.063	12	.007	15	0	1	0	15	-.006	4
179		14	max	946.942	2	.576	2	.126	1	0	2	0	1	-.001	15
180			min	-1385.977	3	-.003	3	.007	15	0	1	0	15	-.006	4
181		15	max	947.463	2	.484	2	.126	1	0	2	0	1	-.001	15
182			min	-1385.587	3	-.073	3	.007	15	0	1	0	15	-.006	4
183		16	max	947.984	2	.391	2	.126	1	0	2	0	1	-.001	15
184			min	-1385.196	3	-.142	3	.007	15	0	1	0	15	-.006	4
185		17	max	948.504	2	.299	2	.126	1	0	2	0	1	-.001	12
186			min	-1384.806	3	-.211	3	.007	15	0	1	0	15	-.006	4
187		18	max	949.025	2	.206	2	.126	1	0	2	0	1	-.001	12
188			min	-1384.415	3	-.281	3	.007	15	0	1	0	15	-.006	4
189		19	max	949.546	2	.113	2	.126	1	0	2	0	1	-.001	12
190			min	-1384.025	3	-.35	3	.007	15	0	1	0	15	-.006	4
191	M3	1	max	872.918	2	7.663	4	.134	1	0	3	0	1	.006	4
192			min	-958.898	3	1.802	15	.008	15	0	1	0	15	.001	12
193		2	max	872.747	2	6.902	4	.134	1	0	3	0	1	.004	2
194			min	-959.025	3	1.623	15	.008	15	0	1	0	15	0	3
195		3	max	872.577	2	6.142	4	.134	1	0	3	0	1	.001	2
196			min	-959.153	3	1.444	15	.008	15	0	1	0	15	-.001	3
197		4	max	872.407	2	5.381	4	.134	1	0	3	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-959.281	3	1.265	15	.008	15	0	1	0	15	-.003	3
199		5	max	872.236	2	4.62	4	.134	1	0	3	0	1	0	15
200			min	-959.409	3	1.086	15	.008	15	0	1	0	15	-.004	4
201		6	max	872.066	2	3.859	4	.134	1	0	3	0	1	-.001	15
202			min	-959.536	3	.907	15	.008	15	0	1	0	15	-.006	4
203		7	max	871.895	2	3.098	4	.134	1	0	3	0	1	-.002	15
204			min	-959.664	3	.728	15	.008	15	0	1	0	15	-.007	4
205		8	max	871.725	2	2.337	4	.134	1	0	3	0	1	-.002	15
206			min	-959.792	3	.549	15	.008	15	0	1	0	15	-.008	4
207		9	max	871.555	2	1.576	4	.134	1	0	3	0	1	-.002	15
208			min	-959.92	3	.37	15	.008	15	0	1	0	15	-.009	4
209		10	max	871.384	2	.815	4	.134	1	0	3	0	1	-.002	15
210			min	-960.047	3	.173	12	.008	15	0	1	0	15	-.01	4
211		11	max	871.214	2	.219	2	.134	1	0	3	0	1	-.002	15
212			min	-960.175	3	-.209	3	.008	15	0	1	0	15	-.01	4
213		12	max	871.044	2	-.166	15	.134	1	0	3	0	1	-.002	15
214			min	-960.303	3	-.707	4	.008	15	0	1	0	15	-.01	4
215		13	max	870.873	2	-.345	15	.134	1	0	3	0	1	-.002	15
216			min	-960.431	3	-1.468	4	.008	15	0	1	0	15	-.009	4
217		14	max	870.703	2	-.524	15	.134	1	0	3	0	1	-.002	15
218			min	-960.558	3	-2.229	4	.008	15	0	1	0	15	-.009	4
219		15	max	870.533	2	-.703	15	.134	1	0	3	0	1	-.002	15
220			min	-960.686	3	-2.99	4	.008	15	0	1	0	15	-.008	4
221		16	max	870.362	2	-.882	15	.134	1	0	3	0	1	-.001	15
222			min	-960.814	3	-3.751	4	.008	15	0	1	0	15	-.006	4
223		17	max	870.192	2	-1.061	15	.134	1	0	3	.001	1	-.001	15
224			min	-960.942	3	-4.512	4	.008	15	0	1	0	15	-.004	4
225		18	max	870.022	2	-1.239	15	.134	1	0	3	.001	1	0	15
226			min	-961.069	3	-5.273	4	.008	15	0	1	0	15	-.002	4
227		19	max	869.851	2	-1.418	15	.134	1	0	3	.001	1	0	1
228			min	-961.197	3	-6.034	4	.008	15	0	1	0	15	0	1
229	M4	1	max	657.777	1	0	1	-.256	15	0	1	.001	1	0	1
230			min	30.481	15	0	1	-4.416	1	0	1	0	15	0	1
231		2	max	657.948	1	0	1	-.256	15	0	1	0	1	0	1
232			min	30.533	15	0	1	-4.416	1	0	1	0	15	0	1
233		3	max	658.118	1	0	1	-.256	15	0	1	0	1	0	1
234			min	30.584	15	0	1	-4.416	1	0	1	0	10	0	1
235		4	max	658.288	1	0	1	-.256	15	0	1	0	15	0	1
236			min	30.636	15	0	1	-4.416	1	0	1	0	1	0	1
237		5	max	658.459	1	0	1	-.256	15	0	1	0	15	0	1
238			min	30.687	15	0	1	-4.416	1	0	1	0	1	0	1
239		6	max	658.629	1	0	1	-.256	15	0	1	0	15	0	1
240			min	30.738	15	0	1	-4.416	1	0	1	-.001	1	0	1
241		7	max	658.799	1	0	1	-.256	15	0	1	0	15	0	1
242			min	30.79	15	0	1	-4.416	1	0	1	-.002	1	0	1
243		8	max	658.97	1	0	1	-.256	15	0	1	0	15	0	1
244			min	30.841	15	0	1	-4.416	1	0	1	-.002	1	0	1
245		9	max	659.14	1	0	1	-.256	15	0	1	0	15	0	1
246			min	30.892	15	0	1	-4.416	1	0	1	-.003	1	0	1
247		10	max	659.31	1	0	1	-.256	15	0	1	0	15	0	1
248			min	30.944	15	0	1	-4.416	1	0	1	-.003	1	0	1
249		11	max	659.481	1	0	1	-.256	15	0	1	0	15	0	1
250			min	30.995	15	0	1	-4.416	1	0	1	-.004	1	0	1
251		12	max	659.651	1	0	1	-.256	15	0	1	0	15	0	1
252			min	31.047	15	0	1	-4.416	1	0	1	-.004	1	0	1
253		13	max	659.821	1	0	1	-.256	15	0	1	0	15	0	1
254			min	31.098	15	0	1	-4.416	1	0	1	-.005	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	659.992	1	0	1	-.256	15	0	1	0	15	0	1
256		min	31.149	15	0	1	-4.416	1	0	1	-.006	1	0	1
257	15	max	660.162	1	0	1	-.256	15	0	1	0	15	0	1
258		min	31.201	15	0	1	-4.416	1	0	1	-.006	1	0	1
259	16	max	660.332	1	0	1	-.256	15	0	1	0	15	0	1
260		min	31.252	15	0	1	-4.416	1	0	1	-.007	1	0	1
261	17	max	660.503	1	0	1	-.256	15	0	1	0	15	0	1
262		min	31.304	15	0	1	-4.416	1	0	1	-.007	1	0	1
263	18	max	660.673	1	0	1	-.256	15	0	1	0	15	0	1
264		min	31.355	15	0	1	-4.416	1	0	1	-.008	1	0	1
265	19	max	660.843	1	0	1	-.256	15	0	1	0	15	0	1
266		min	31.406	15	0	1	-4.416	1	0	1	-.008	1	0	1
267	M6	1	max	2805.059	2	2.212	2	0	1	0	0	1	0	1
268		min	-4276.497	3	.283	12	0	1	0	1	0	1	0	1
269	2	max	2805.58	2	2.12	2	0	1	0	1	0	1	0	12
270		min	-4276.106	3	.237	12	0	1	0	1	0	1	0	2
271	3	max	2806.101	2	2.027	2	0	1	0	1	0	1	0	12
272		min	-4275.716	3	.191	12	0	1	0	1	0	1	-.002	2
273	4	max	2806.621	2	1.934	2	0	1	0	1	0	1	0	12
274		min	-4275.325	3	.133	3	0	1	0	1	0	1	-.002	2
275	5	max	2807.142	2	1.842	2	0	1	0	1	0	1	0	12
276		min	-4274.935	3	.063	3	0	1	0	1	0	1	-.003	2
277	6	max	2807.663	2	1.749	2	0	1	0	1	0	1	0	3
278		min	-4274.544	3	-.006	3	0	1	0	1	0	1	-.004	2
279	7	max	2808.183	2	1.657	2	0	1	0	1	0	1	0	3
280		min	-4274.154	3	-.076	3	0	1	0	1	0	1	-.004	2
281	8	max	2808.704	2	1.564	2	0	1	0	1	0	1	0	3
282		min	-4273.763	3	-.145	3	0	1	0	1	0	1	-.005	2
283	9	max	2809.225	2	1.471	2	0	1	0	1	0	1	0	3
284		min	-4273.372	3	-.214	3	0	1	0	1	0	1	-.005	2
285	10	max	2809.745	2	1.379	2	0	1	0	1	0	1	0	3
286		min	-4272.982	3	-.284	3	0	1	0	1	0	1	-.006	2
287	11	max	2810.266	2	1.286	2	0	1	0	1	0	1	0	3
288		min	-4272.591	3	-.353	3	0	1	0	1	0	1	-.006	2
289	12	max	2810.787	2	1.193	2	0	1	0	1	0	1	0	3
290		min	-4272.201	3	-.423	3	0	1	0	1	0	1	-.007	2
291	13	max	2811.308	2	1.101	2	0	1	0	1	0	1	0	3
292		min	-4271.81	3	-.492	3	0	1	0	1	0	1	-.007	2
293	14	max	2811.828	2	1.008	2	0	1	0	1	0	1	0	3
294		min	-4271.42	3	-.562	3	0	1	0	1	0	1	-.007	2
295	15	max	2812.349	2	.916	2	0	1	0	1	0	1	0	3
296		min	-4271.029	3	-.631	3	0	1	0	1	0	1	-.008	2
297	16	max	2812.87	2	.823	2	0	1	0	1	0	1	0	3
298		min	-4270.639	3	-.701	3	0	1	0	1	0	1	-.008	2
299	17	max	2813.39	2	.73	2	0	1	0	1	0	1	.001	3
300		min	-4270.248	3	-.77	3	0	1	0	1	0	1	-.008	2
301	18	max	2813.911	2	.638	2	0	1	0	1	0	1	.002	3
302		min	-4269.858	3	-.84	3	0	1	0	1	0	1	-.009	2
303	19	max	2814.432	2	.545	2	0	1	0	1	0	1	.002	3
304		min	-4269.467	3	-.909	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	2755.044	2	7.679	4	0	1	0	0	1	.009	2
306		min	-2811.984	3	1.804	15	0	1	0	1	0	1	-.002	3
307	2	max	2754.874	2	6.918	4	0	1	0	1	0	1	.006	2
308		min	-2812.111	3	1.625	15	0	1	0	1	0	1	-.003	3
309	3	max	2754.703	2	6.157	4	0	1	0	1	0	1	.004	2
310		min	-2812.239	3	1.446	15	0	1	0	1	0	1	-.005	3
311	4	max	2754.533	2	5.396	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2812.367	3	1.267	15	0	1	0	1	0	1	-.006	3
313	5	max	2754.363	2	4.635	4	0	1	0	1	0	1	0	2
314		min	-2812.495	3	1.089	15	0	1	0	1	0	1	-.007	3
315	6	max	2754.192	2	3.874	4	0	1	0	1	0	1	-.001	15
316		min	-2812.622	3	.91	15	0	1	0	1	0	1	-.007	3
317	7	max	2754.022	2	3.113	4	0	1	0	1	0	1	-.002	15
318		min	-2812.75	3	.731	15	0	1	0	1	0	1	-.008	3
319	8	max	2753.852	2	2.368	2	0	1	0	1	0	1	-.002	15
320		min	-2812.878	3	.479	12	0	1	0	1	0	1	-.008	4
321	9	max	2753.681	2	1.775	2	0	1	0	1	0	1	-.002	15
322		min	-2813.006	3	.182	12	0	1	0	1	0	1	-.009	4
323	10	max	2753.511	2	1.182	2	0	1	0	1	0	1	-.002	15
324		min	-2813.133	3	-.243	3	0	1	0	1	0	1	-.01	4
325	11	max	2753.34	2	.589	2	0	1	0	1	0	1	-.002	15
326		min	-2813.261	3	-.688	3	0	1	0	1	0	1	-.01	4
327	12	max	2753.17	2	-.004	2	0	1	0	1	0	1	-.002	15
328		min	-2813.389	3	-1.133	3	0	1	0	1	0	1	-.01	4
329	13	max	2753	2	-.342	15	0	1	0	1	0	1	-.002	15
330		min	-2813.517	3	-1.578	3	0	1	0	1	0	1	-.009	4
331	14	max	2752.829	2	-.521	15	0	1	0	1	0	1	-.002	15
332		min	-2813.644	3	-2.214	4	0	1	0	1	0	1	-.009	4
333	15	max	2752.659	2	-.7	15	0	1	0	1	0	1	-.002	15
334		min	-2813.772	3	-2.975	4	0	1	0	1	0	1	-.007	4
335	16	max	2752.489	2	-.879	15	0	1	0	1	0	1	-.001	15
336		min	-2813.9	3	-3.736	4	0	1	0	1	0	1	-.006	4
337	17	max	2752.318	2	-1.058	15	0	1	0	1	0	1	-.001	15
338		min	-2814.028	3	-4.497	4	0	1	0	1	0	1	-.004	4
339	18	max	2752.148	2	-1.237	15	0	1	0	1	0	1	0	15
340		min	-2814.156	3	-5.258	4	0	1	0	1	0	1	-.002	4
341	19	max	2751.978	2	-1.416	15	0	1	0	1	0	1	0	1
342		min	-2814.283	3	-6.019	4	0	1	0	1	0	1	0	1
343	M8	1	max	1769.834	2	0	1	0	1	0	1	0	1	1
344		min	61.497	15	0	1	0	1	0	1	0	1	0	1
345	2	max	1770.005	2	0	1	0	1	0	1	0	1	0	1
346		min	61.549	15	0	1	0	1	0	1	0	1	0	1
347	3	max	1770.175	2	0	1	0	1	0	1	0	1	0	1
348		min	61.6	15	0	1	0	1	0	1	0	1	0	1
349	4	max	1770.345	2	0	1	0	1	0	1	0	1	0	1
350		min	61.651	15	0	1	0	1	0	1	0	1	0	1
351	5	max	1770.516	2	0	1	0	1	0	1	0	1	0	1
352		min	61.703	15	0	1	0	1	0	1	0	1	0	1
353	6	max	1770.686	2	0	1	0	1	0	1	0	1	0	1
354		min	61.754	15	0	1	0	1	0	1	0	1	0	1
355	7	max	1770.856	2	0	1	0	1	0	1	0	1	0	1
356		min	61.805	15	0	1	0	1	0	1	0	1	0	1
357	8	max	1771.027	2	0	1	0	1	0	1	0	1	0	1
358		min	61.857	15	0	1	0	1	0	1	0	1	0	1
359	9	max	1771.197	2	0	1	0	1	0	1	0	1	0	1
360		min	61.908	15	0	1	0	1	0	1	0	1	0	1
361	10	max	1771.367	2	0	1	0	1	0	1	0	1	0	1
362		min	61.96	15	0	1	0	1	0	1	0	1	0	1
363	11	max	1771.538	2	0	1	0	1	0	1	0	1	0	1
364		min	62.011	15	0	1	0	1	0	1	0	1	0	1
365	12	max	1771.708	2	0	1	0	1	0	1	0	1	0	1
366		min	62.062	15	0	1	0	1	0	1	0	1	0	1
367	13	max	1771.878	2	0	1	0	1	0	1	0	1	0	1
368		min	62.114	15	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	1772.049	2	0	1	0	1	0	1	0	1	0	1
370			min	62.165	15	0	1	0	1	0	1	0	1	0	1
371		15	max	1772.219	2	0	1	0	1	0	1	0	1	0	1
372			min	62.217	15	0	1	0	1	0	1	0	1	0	1
373		16	max	1772.389	2	0	1	0	1	0	1	0	1	0	1
374			min	62.268	15	0	1	0	1	0	1	0	1	0	1
375		17	max	1772.56	2	0	1	0	1	0	1	0	1	0	1
376			min	62.319	15	0	1	0	1	0	1	0	1	0	1
377		18	max	1772.73	2	0	1	0	1	0	1	0	1	0	1
378			min	62.371	15	0	1	0	1	0	1	0	1	0	1
379		19	max	1772.9	2	0	1	0	1	0	1	0	1	0	1
380			min	62.422	15	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	940.173	2	2.02	4	-0.007	15	0	1	0	2	0	1
382			min	-1391.054	3	.475	15	-.126	1	0	2	0	3	0	1
383		2	max	940.694	2	1.901	4	-.007	15	0	1	0	10	0	15
384			min	-1390.664	3	.447	15	-.126	1	0	2	0	1	0	4
385		3	max	941.215	2	1.782	4	-.007	15	0	1	0	10	0	15
386			min	-1390.273	3	.419	15	-.126	1	0	2	0	1	-.001	4
387		4	max	941.735	2	1.664	4	-.007	15	0	1	0	15	0	15
388			min	-1389.883	3	.391	15	-.126	1	0	2	0	1	-.002	4
389		5	max	942.256	2	1.545	4	-.007	15	0	1	0	15	0	15
390			min	-1389.492	3	.363	15	-.126	1	0	2	0	1	-.003	4
391		6	max	942.777	2	1.426	4	-.007	15	0	1	0	15	0	15
392			min	-1389.102	3	.335	15	-.126	1	0	2	0	1	-.003	4
393		7	max	943.298	2	1.307	4	-.007	15	0	1	0	15	0	15
394			min	-1388.711	3	.307	15	-.126	1	0	2	0	1	-.004	4
395		8	max	943.818	2	1.188	4	-.007	15	0	1	0	15	0	15
396			min	-1388.321	3	.279	15	-.126	1	0	2	0	1	-.004	4
397		9	max	944.339	2	1.069	4	-.007	15	0	1	0	15	-.001	15
398			min	-1387.93	3	.248	12	-.126	1	0	2	0	1	-.004	4
399		10	max	944.86	2	.95	4	-.007	15	0	1	0	15	-.001	15
400			min	-1387.54	3	.202	12	-.126	1	0	2	0	1	-.005	4
401		11	max	945.38	2	.854	2	-.007	15	0	1	0	15	-.001	15
402			min	-1387.149	3	.156	12	-.126	1	0	2	0	1	-.005	4
403		12	max	945.901	2	.762	2	-.007	15	0	1	0	15	-.001	15
404			min	-1386.758	3	.109	12	-.126	1	0	2	0	1	-.005	4
405		13	max	946.422	2	.669	2	-.007	15	0	1	0	15	-.001	15
406			min	-1386.368	3	.063	12	-.126	1	0	2	0	1	-.006	4
407		14	max	946.942	2	.576	2	-.007	15	0	1	0	15	-.001	15
408			min	-1385.977	3	-.003	3	-.126	1	0	2	0	1	-.006	4
409		15	max	947.463	2	.484	2	-.007	15	0	1	0	15	-.001	15
410			min	-1385.587	3	-.073	3	-.126	1	0	2	0	1	-.006	4
411		16	max	947.984	2	.391	2	-.007	15	0	1	0	15	-.001	15
412			min	-1385.196	3	-.142	3	-.126	1	0	2	0	1	-.006	4
413		17	max	948.504	2	.299	2	-.007	15	0	1	0	15	-.001	12
414			min	-1384.806	3	-.211	3	-.126	1	0	2	0	1	-.006	4
415		18	max	949.025	2	.206	2	-.007	15	0	1	0	15	-.001	12
416			min	-1384.415	3	-.281	3	-.126	1	0	2	0	1	-.006	4
417		19	max	949.546	2	.113	2	-.007	15	0	1	0	15	-.001	12
418			min	-1384.025	3	-.35	3	-.126	1	0	2	0	1	-.006	4
419	M11	1	max	872.918	2	7.663	4	-.008	15	0	1	0	15	.006	4
420			min	-958.898	3	1.802	15	-.134	1	0	3	0	1	.001	12
421		2	max	872.747	2	6.902	4	-.008	15	0	1	0	15	.004	2
422			min	-959.025	3	1.623	15	-.134	1	0	3	0	1	0	3
423		3	max	872.577	2	6.142	4	-.008	15	0	1	0	15	.001	2
424			min	-959.153	3	1.444	15	-.134	1	0	3	0	1	-.001	3
425		4	max	872.407	2	5.381	4	-.008	15	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426		min	-959.281	3	1.265	15	-.134	1	0	3	0	1	-.003	3
427	5	max	872.236	2	4.62	4	-.008	15	0	1	0	15	0	15
428		min	-959.409	3	1.086	15	-.134	1	0	3	0	1	-.004	4
429	6	max	872.066	2	3.859	4	-.008	15	0	1	0	15	-.001	15
430		min	-959.536	3	.907	15	-.134	1	0	3	0	1	-.006	4
431	7	max	871.895	2	3.098	4	-.008	15	0	1	0	15	-.002	15
432		min	-959.664	3	.728	15	-.134	1	0	3	0	1	-.007	4
433	8	max	871.725	2	2.337	4	-.008	15	0	1	0	15	-.002	15
434		min	-959.792	3	.549	15	-.134	1	0	3	0	1	-.008	4
435	9	max	871.555	2	1.576	4	-.008	15	0	1	0	15	-.002	15
436		min	-959.92	3	.37	15	-.134	1	0	3	0	1	-.009	4
437	10	max	871.384	2	.815	4	-.008	15	0	1	0	15	-.002	15
438		min	-960.047	3	.173	12	-.134	1	0	3	0	1	-.01	4
439	11	max	871.214	2	.219	2	-.008	15	0	1	0	15	-.002	15
440		min	-960.175	3	-.209	3	-.134	1	0	3	0	1	-.01	4
441	12	max	871.044	2	-.166	15	-.008	15	0	1	0	15	-.002	15
442		min	-960.303	3	-.707	4	-.134	1	0	3	0	1	-.01	4
443	13	max	870.873	2	-.345	15	-.008	15	0	1	0	15	-.002	15
444		min	-960.431	3	-1.468	4	-.134	1	0	3	0	1	-.009	4
445	14	max	870.703	2	-.524	15	-.008	15	0	1	0	15	-.002	15
446		min	-960.558	3	-2.229	4	-.134	1	0	3	0	1	-.009	4
447	15	max	870.533	2	-.703	15	-.008	15	0	1	0	15	-.002	15
448		min	-960.686	3	-2.99	4	-.134	1	0	3	0	1	-.008	4
449	16	max	870.362	2	-.882	15	-.008	15	0	1	0	15	-.001	15
450		min	-960.814	3	-3.751	4	-.134	1	0	3	0	1	-.006	4
451	17	max	870.192	2	-1.061	15	-.008	15	0	1	0	15	-.001	15
452		min	-960.942	3	-4.512	4	-.134	1	0	3	-.001	1	-.004	4
453	18	max	870.022	2	-1.239	15	-.008	15	0	1	0	15	0	15
454		min	-961.069	3	-5.273	4	-.134	1	0	3	-.001	1	-.002	4
455	19	max	869.851	2	-1.418	15	-.008	15	0	1	0	15	0	1
456		min	-961.197	3	-6.034	4	-.134	1	0	3	-.001	1	0	1
457	M12	1	max	657.777	1	0	4.416	1	0	1	0	15	0	1
458		min	30.481	15	0	1	.256	15	0	1	-.001	1	0	1
459	2	max	657.948	1	0	1	4.416	1	0	1	0	15	0	1
460		min	30.533	15	0	1	.256	15	0	1	0	1	0	1
461	3	max	658.118	1	0	1	4.416	1	0	1	0	10	0	1
462		min	30.584	15	0	1	.256	15	0	1	0	1	0	1
463	4	max	658.288	1	0	1	4.416	1	0	1	0	1	0	1
464		min	30.636	15	0	1	.256	15	0	1	0	15	0	1
465	5	max	658.459	1	0	1	4.416	1	0	1	0	1	0	1
466		min	30.687	15	0	1	.256	15	0	1	0	15	0	1
467	6	max	658.629	1	0	1	4.416	1	0	1	.001	1	0	1
468		min	30.738	15	0	1	.256	15	0	1	0	15	0	1
469	7	max	658.799	1	0	1	4.416	1	0	1	.002	1	0	1
470		min	30.79	15	0	1	.256	15	0	1	0	15	0	1
471	8	max	658.97	1	0	1	4.416	1	0	1	.002	1	0	1
472		min	30.841	15	0	1	.256	15	0	1	0	15	0	1
473	9	max	659.14	1	0	1	4.416	1	0	1	.003	1	0	1
474		min	30.892	15	0	1	.256	15	0	1	0	15	0	1
475	10	max	659.31	1	0	1	4.416	1	0	1	.003	1	0	1
476		min	30.944	15	0	1	.256	15	0	1	0	15	0	1
477	11	max	659.481	1	0	1	4.416	1	0	1	.004	1	0	1
478		min	30.995	15	0	1	.256	15	0	1	0	15	0	1
479	12	max	659.651	1	0	1	4.416	1	0	1	.004	1	0	1
480		min	31.047	15	0	1	.256	15	0	1	0	15	0	1
481	13	max	659.821	1	0	1	4.416	1	0	1	.005	1	0	1
482		min	31.098	15	0	1	.256	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	659.992	1	0	1	4.416	1	0	1	.006	1	0	1
484			min	31.149	15	0	1	.256	15	0	1	0	15	0	1
485		15	max	660.162	1	0	1	4.416	1	0	1	.006	1	0	1
486			min	31.201	15	0	1	.256	15	0	1	0	15	0	1
487		16	max	660.332	1	0	1	4.416	1	0	1	.007	1	0	1
488			min	31.252	15	0	1	.256	15	0	1	0	15	0	1
489		17	max	660.503	1	0	1	4.416	1	0	1	.007	1	0	1
490			min	31.304	15	0	1	.256	15	0	1	0	15	0	1
491		18	max	660.673	1	0	1	4.416	1	0	1	.008	1	0	1
492			min	31.355	15	0	1	.256	15	0	1	0	15	0	1
493		19	max	660.843	1	0	1	4.416	1	0	1	.008	1	0	1
494			min	31.406	15	0	1	.256	15	0	1	0	15	0	1
495	M1	1	max	107.753	1	705.885	3	-2.213	15	0	2	.089	1	0	15
496			min	6.065	15	-359.341	2	-37.678	1	0	3	.005	15	-.011	2
497		2	max	108.574	1	705.005	3	-2.213	15	0	2	.069	1	.179	2
498			min	6.313	15	-360.515	2	-37.678	1	0	3	.004	15	-.376	3
499		3	max	601.832	3	487.714	2	-2.206	15	0	3	.05	1	.359	2
500			min	-348.628	2	-561.708	3	-37.59	1	0	2	.003	15	-.733	3
501		4	max	602.448	3	486.541	2	-2.206	15	0	3	.03	1	.102	2
502			min	-347.806	2	-562.588	3	-37.59	1	0	2	.002	15	-.436	3
503		5	max	603.065	3	485.368	2	-2.206	15	0	3	.01	1	-.003	15
504			min	-346.985	2	-563.468	3	-37.59	1	0	2	0	15	-.154	2
505		6	max	603.681	3	484.194	2	-2.206	15	0	3	0	15	.159	3
506			min	-346.163	2	-564.348	3	-37.59	1	0	2	-.01	1	-.41	2
507		7	max	604.297	3	483.021	2	-2.206	15	0	3	-.002	15	.457	3
508			min	-345.341	2	-565.228	3	-37.59	1	0	2	-.03	1	-.665	2
509		8	max	604.913	3	481.847	2	-2.206	15	0	3	-.003	15	.755	3
510			min	-344.52	2	-566.108	3	-37.59	1	0	2	-.05	1	-.92	2
511		9	max	620.13	3	51.767	2	-3.584	15	0	9	.032	1	.875	3
512			min	-297.497	2	.359	15	-61.378	1	0	3	.002	15	-1.05	2
513		10	max	620.746	3	50.593	2	-3.584	15	0	9	0	10	.859	3
514			min	-296.676	2	.005	15	-61.378	1	0	3	0	1	-1.077	2
515		11	max	621.362	3	49.42	2	-3.584	15	0	9	-.002	15	.843	3
516			min	-295.854	2	-1.454	4	-61.378	1	0	3	-.033	1	-1.103	2
517		12	max	636.155	3	393.096	3	-2.157	15	0	2	.049	1	.74	3
518			min	-248.623	2	-592.063	2	-37.156	1	0	3	.003	15	-.981	2
519		13	max	636.772	3	392.216	3	-2.157	15	0	2	.03	1	.533	3
520			min	-247.802	2	-593.236	2	-37.156	1	0	3	.002	15	-.669	2
521		14	max	637.388	3	391.336	3	-2.157	15	0	2	.01	1	.326	3
522			min	-246.98	2	-594.41	2	-37.156	1	0	3	0	15	-.355	2
523		15	max	638.004	3	390.456	3	-2.157	15	0	2	0	15	.12	3
524			min	-246.159	2	-595.583	2	-37.156	1	0	3	-.009	1	-.045	1
525		16	max	638.62	3	389.576	3	-2.157	15	0	2	-.002	15	.273	2
526			min	-245.337	2	-596.757	2	-37.156	1	0	3	-.029	1	-.086	3
527		17	max	639.236	3	388.696	3	-2.157	15	0	2	-.003	15	.588	2
528			min	-244.515	2	-597.93	2	-37.156	1	0	3	-.049	1	-.291	3
529		18	max	-6.32	15	575.738	2	-2.405	15	0	3	-.004	15	.299	2
530			min	-109.073	1	-293.393	3	-41.374	1	0	2	-.069	1	-.145	3
531		19	max	-6.072	15	574.565	2	-2.405	15	0	3	-.005	15	.01	3
532			min	-108.252	1	-294.274	3	-41.374	1	0	2	-.091	1	-.005	2
533	M5	1	max	247.63	1	2316.266	3	0	1	0	1	0	1	.022	2
534			min	7.528	12	-1249.863	2	0	1	0	1	0	1	0	15
535		2	max	248.451	1	2315.386	3	0	1	0	1	0	1	.682	2
536			min	7.939	12	-1251.036	2	0	1	0	1	0	1	-1.214	3
537		3	max	1810.984	3	1267.63	2	0	1	0	1	0	1	1.312	2
538			min	-1064.561	2	-1594.901	3	0	1	0	1	0	1	-2.389	3
539		4	max	1811.6	3	1266.457	2	0	1	0	1	0	1	.643	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1063.74	2	-1595.781	3	0	1	0	1	0	1	-1.547	3
541		5	max	1812.216	3	1265.283	2	0	1	0	1	0	1	.017	9
542			min	-1062.918	2	-1596.661	3	0	1	0	1	0	1	-.705	3
543		6	max	1812.832	3	1264.11	2	0	1	0	1	0	1	.138	3
544			min	-1062.096	2	-1597.541	3	0	1	0	1	0	1	-.692	2
545		7	max	1813.448	3	1262.937	2	0	1	0	1	0	1	.981	3
546			min	-1061.275	2	-1598.421	3	0	1	0	1	0	1	-1.359	2
547		8	max	1814.065	3	1261.763	2	0	1	0	1	0	1	1.825	3
548			min	-1060.453	2	-1599.301	3	0	1	0	1	0	1	-2.025	2
549		9	max	1823.901	3	176.742	2	0	1	0	1	0	1	2.099	3
550			min	-948.921	2	.349	15	0	1	0	1	0	1	-2.323	2
551		10	max	1824.517	3	175.569	2	0	1	0	1	0	1	2.031	3
552			min	-948.1	2	-.006	5	0	1	0	1	0	1	-2.416	2
553		11	max	1825.133	3	174.396	2	0	1	0	1	0	1	1.965	3
554			min	-947.278	2	-1.46	4	0	1	0	1	0	1	-2.508	2
555		12	max	1835.816	3	1067.583	3	0	1	0	1	0	1	1.721	3
556			min	-836.163	2	-1615.427	2	0	1	0	1	0	1	-2.248	2
557		13	max	1836.432	3	1066.703	3	0	1	0	1	0	1	1.158	3
558			min	-835.341	2	-1616.601	2	0	1	0	1	0	1	-1.396	2
559		14	max	1837.048	3	1065.823	3	0	1	0	1	0	1	.595	3
560			min	-834.52	2	-1617.774	2	0	1	0	1	0	1	-.542	2
561		15	max	1837.665	3	1064.942	3	0	1	0	1	0	1	.312	2
562			min	-833.698	2	-1618.947	2	0	1	0	1	0	1	0	15
563		16	max	1838.281	3	1064.062	3	0	1	0	1	0	1	1.166	2
564			min	-832.877	2	-1620.121	2	0	1	0	1	0	1	-.529	3
565		17	max	1838.897	3	1063.182	3	0	1	0	1	0	1	2.021	2
566			min	-832.055	2	-1621.294	2	0	1	0	1	0	1	-1.09	3
567		18	max	-9.6	12	1946.633	2	0	1	0	1	0	1	1.037	2
568			min	-247.454	1	-1037.473	3	0	1	0	1	0	1	-.568	3
569		19	max	-9.189	12	1945.459	2	0	1	0	1	0	1	.01	2
570			min	-246.632	1	-1038.353	3	0	1	0	1	0	1	-.021	3
571	M9	1	max	107.753	1	705.885	3	37.678	1	0	3	-.005	15	0	15
572			min	6.065	15	-359.341	2	2.213	15	0	2	-.089	1	-.011	2
573		2	max	108.574	1	705.005	3	37.678	1	0	3	-.004	15	.179	2
574			min	6.313	15	-360.515	2	2.213	15	0	2	-.069	1	-.376	3
575		3	max	601.832	3	487.714	2	37.59	1	0	2	-.003	15	.359	2
576			min	-348.628	2	-561.708	3	2.206	15	0	3	-.05	1	-.733	3
577		4	max	602.448	3	486.541	2	37.59	1	0	2	-.002	15	.102	2
578			min	-347.806	2	-562.588	3	2.206	15	0	3	-.03	1	-.436	3
579		5	max	603.065	3	485.368	2	37.59	1	0	2	0	15	-.003	15
580			min	-346.985	2	-563.468	3	2.206	15	0	3	-.01	1	-.154	2
581		6	max	603.681	3	484.194	2	37.59	1	0	2	.01	1	.159	3
582			min	-346.163	2	-564.348	3	2.206	15	0	3	0	15	-.41	2
583		7	max	604.297	3	483.021	2	37.59	1	0	2	.03	1	.457	3
584			min	-345.341	2	-565.228	3	2.206	15	0	3	.002	15	-.665	2
585		8	max	604.913	3	481.847	2	37.59	1	0	2	.05	1	.755	3
586			min	-344.52	2	-566.108	3	2.206	15	0	3	.003	15	-.92	2
587		9	max	620.13	3	51.767	2	61.378	1	0	3	-.002	15	.875	3
588			min	-297.497	2	.359	15	3.584	15	0	9	-.032	1	-1.05	2
589		10	max	620.746	3	50.593	2	61.378	1	0	3	0	1	.859	3
590			min	-296.676	2	.005	15	3.584	15	0	9	0	10	-1.077	2
591		11	max	621.362	3	49.42	2	61.378	1	0	3	.033	1	.843	3
592			min	-295.854	2	-1.454	4	3.584	15	0	9	.002	15	-1.103	2
593		12	max	636.155	3	393.096	3	37.156	1	0	3	-.003	15	.74	3
594			min	-248.623	2	-592.063	2	2.157	15	0	2	-.049	1	-.981	2
595		13	max	636.772	3	392.216	3	37.156	1	0	3	-.002	15	.533	3
596			min	-247.802	2	-593.236	2	2.157	15	0	2	-.03	1	-.669	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	637.388	3	391.336	3	37.156	1	0	3	0	15	.326	3
598		min	-246.98	2	-594.41	2	2.157	15	0	2	-.01	1	-.355	2
599	15	max	638.004	3	390.456	3	37.156	1	0	3	.009	1	.12	3
600		min	-246.159	2	-595.583	2	2.157	15	0	2	0	15	-.045	1
601	16	max	638.62	3	389.576	3	37.156	1	0	3	.029	1	.273	2
602		min	-245.337	2	-596.757	2	2.157	15	0	2	.002	15	-.086	3
603	17	max	639.236	3	388.696	3	37.156	1	0	3	.049	1	.588	2
604		min	-244.515	2	-597.93	2	2.157	15	0	2	.003	15	-.291	3
605	18	max	-6.32	15	575.738	2	41.374	1	0	2	.069	1	.299	2
606		min	-109.073	1	-293.393	3	2.405	15	0	3	.004	15	-.145	3
607	19	max	-6.072	15	574.565	2	41.374	1	0	2	.091	1	.01	3
608		min	-108.252	1	-294.274	3	2.405	15	0	3	.005	15	-.005	2

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.116	2	.01	3	9.83e-3	2	NC	1	NC	1
2			min	0	15	-.035	3	-.007	2	-3.403e-3	3	NC	1	NC	1
3		2	max	0	1	.084	2	.012	3	1.05e-2	2	NC	4	NC	1
4			min	0	15	.002	15	-.004	2	-3.163e-3	3	1709.54	3	NC	1
5		3	max	0	1	.125	3	.015	3	1.118e-2	2	NC	4	NC	2
6			min	0	15	.001	15	-.003	10	-2.923e-3	3	937.877	3	9676.171	1
7		4	max	0	1	.171	3	.021	1	1.185e-2	2	NC	4	NC	2
8			min	0	15	0	15	-.003	10	-2.682e-3	3	727.894	3	6686.093	1
9		5	max	0	1	.186	3	.024	1	1.253e-2	2	NC	4	NC	2
10			min	0	15	0	15	-.004	10	-2.442e-3	3	678.249	3	5935.452	1
11	6	max	0	1	.171	3	.024	3	1.32e-2	2	NC	4	NC	2	
12		min	0	15	.001	15	-.005	10	-2.202e-3	3	729.249	3	6502.281	1	
13	7	max	0	1	.131	3	.027	3	1.388e-2	2	NC	1	NC	2	
14		min	0	15	.002	15	-.008	2	-1.962e-3	3	905.344	3	9021.538	3	
15	8	max	0	1	.138	2	.028	3	1.455e-2	2	NC	1	NC	1	
16		min	0	15	.002	15	-.014	2	-1.722e-3	3	1331.603	3	8219.385	3	
17	9	max	0	1	.168	2	.029	3	1.523e-2	2	NC	4	NC	1	
18		min	0	15	.003	15	-.019	2	-1.482e-3	3	2358.54	3	7837.657	3	
19	10	max	0	1	.182	2	.03	3	1.59e-2	2	NC	4	NC	1	
20		min	0	1	.003	15	-.021	2	-1.242e-3	3	2280.81	2	7732.882	3	
21	11	max	0	15	.168	2	.029	3	1.523e-2	2	NC	4	NC	1	
22		min	0	1	.003	15	-.019	2	-1.482e-3	3	2358.54	3	7837.657	3	
23	12	max	0	15	.138	2	.028	3	1.455e-2	2	NC	1	NC	1	
24		min	0	1	.002	15	-.014	2	-1.722e-3	3	1331.603	3	8219.385	3	
25	13	max	0	15	.131	3	.027	3	1.388e-2	2	NC	1	NC	2	
26		min	0	1	.002	15	-.008	2	-1.962e-3	3	905.344	3	9021.538	3	
27	14	max	0	15	.171	3	.024	3	1.32e-2	2	NC	4	NC	2	
28		min	0	1	.001	15	-.005	10	-2.202e-3	3	729.249	3	6502.281	1	
29	15	max	0	15	.186	3	.024	1	1.253e-2	2	NC	4	NC	2	
30		min	0	1	0	15	-.004	10	-2.442e-3	3	678.249	3	5935.452	1	
31	16	max	0	15	.171	3	.021	1	1.185e-2	2	NC	4	NC	2	
32		min	0	1	0	15	-.003	10	-2.682e-3	3	727.894	3	6686.093	1	
33	17	max	0	15	.125	3	.015	3	1.118e-2	2	NC	4	NC	2	
34		min	0	1	.001	15	-.003	10	-2.923e-3	3	937.877	3	9676.171	1	
35	18	max	0	15	.084	2	.012	3	1.05e-2	2	NC	4	NC	1	
36		min	0	1	.002	15	-.004	2	-3.163e-3	3	1709.54	3	NC	1	
37	19	max	0	15	.116	2	.01	3	9.83e-3	2	NC	1	NC	1	
38		min	0	1	-.035	3	-.007	2	-3.403e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.275	3	.009	3	5.297e-3	2	NC	1	NC	1
40			min	0	15	-.36	2	-.006	2	-4.589e-3	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.394	3	.01	3	6.024e-3	2	NC	4	NC	1
42			min	0	15	-.469	2	-.005	2	-5.284e-3	3	1257.594	3	NC	1
43		3	max	0	1	.5	3	.012	3	6.752e-3	2	NC	5	NC	1
44			min	0	15	-.57	2	-.003	10	-5.98e-3	3	665.965	3	NC	1
45		4	max	0	1	.583	3	.017	1	7.48e-3	2	NC	5	NC	2
46			min	0	15	-.653	2	-.003	10	-6.676e-3	3	486.258	3	8239.933	1
47		5	max	0	1	.639	3	.02	1	8.208e-3	2	NC	5	NC	2
48			min	0	15	-.715	2	-.004	10	-7.371e-3	3	412.25	3	6983.862	1
49		6	max	0	1	.665	3	.021	3	8.936e-3	2	NC	5	NC	2
50			min	0	15	-.755	2	-.005	10	-8.067e-3	3	379.693	2	7422.654	1
51		7	max	0	1	.666	3	.023	3	9.663e-3	2	NC	5	NC	1
52			min	0	15	-.773	2	-.008	2	-8.763e-3	3	362.446	2	NC	1
53		8	max	0	1	.65	3	.025	3	1.039e-2	2	NC	5	NC	1
54			min	0	15	-.776	2	-.013	2	-9.459e-3	3	360.295	2	9351.993	3
55		9	max	0	1	.628	3	.026	3	1.112e-2	2	NC	5	NC	1
56			min	0	15	-.769	2	-.017	2	-1.015e-2	3	365.948	2	8852.003	3
57		10	max	0	1	.616	3	.026	3	1.185e-2	2	NC	5	NC	1
58			min	0	1	-.764	2	-.019	2	-1.085e-2	3	370.464	2	8712.086	3
59		11	max	0	15	.628	3	.026	3	1.112e-2	2	NC	5	NC	1
60			min	0	1	-.769	2	-.017	2	-1.015e-2	3	365.948	2	8852.003	3
61		12	max	0	15	.65	3	.025	3	1.039e-2	2	NC	5	NC	1
62			min	0	1	-.776	2	-.013	2	-9.459e-3	3	360.295	2	9351.993	3
63		13	max	0	15	.666	3	.023	3	9.663e-3	2	NC	5	NC	1
64			min	0	1	-.773	2	-.008	2	-8.763e-3	3	362.446	2	NC	1
65		14	max	0	15	.665	3	.021	3	8.936e-3	2	NC	5	NC	2
66			min	0	1	-.755	2	-.005	10	-8.067e-3	3	379.693	2	7422.654	1
67		15	max	0	15	.639	3	.02	1	8.208e-3	2	NC	5	NC	2
68			min	0	1	-.715	2	-.004	10	-7.371e-3	3	412.25	3	6983.862	1
69		16	max	0	15	.583	3	.017	1	7.48e-3	2	NC	5	NC	2
70			min	0	1	-.653	2	-.003	10	-6.676e-3	3	486.258	3	8239.933	1
71		17	max	0	15	.5	3	.012	3	6.752e-3	2	NC	5	NC	1
72			min	0	1	-.57	2	-.003	10	-5.98e-3	3	665.965	3	NC	1
73		18	max	0	15	.394	3	.01	3	6.024e-3	2	NC	4	NC	1
74			min	0	1	-.469	2	-.005	2	-5.284e-3	3	1257.594	3	NC	1
75		19	max	0	15	.275	3	.009	3	5.297e-3	2	NC	1	NC	1
76			min	0	1	-.36	2	-.006	2	-4.589e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.279	3	.008	3	4.088e-3	3	NC	1	NC	1
78			min	0	1	-.358	2	-.006	2	-5.575e-3	2	NC	1	NC	1
79		2	max	0	15	.371	3	.009	3	4.708e-3	3	NC	4	NC	1
80			min	0	1	-.493	2	-.004	2	-6.35e-3	2	1117.52	2	NC	1
81		3	max	0	15	.456	3	.012	3	5.328e-3	3	NC	5	NC	1
82			min	0	1	-.612	2	-.003	10	-7.125e-3	2	590.475	2	NC	1
83		4	max	0	15	.526	3	.017	1	5.949e-3	3	NC	5	NC	2
84			min	0	1	-.708	2	-.003	10	-7.9e-3	2	429.57	2	8185.068	1
85		5	max	0	15	.579	3	.021	1	6.569e-3	3	NC	5	NC	2
86			min	0	1	-.772	2	-.003	10	-8.675e-3	2	362.302	2	6927.857	1
87		6	max	0	15	.613	3	.02	3	7.189e-3	3	NC	5	NC	2
88			min	0	1	-.806	2	-.005	10	-9.449e-3	2	335.25	2	7341.142	1
89		7	max	0	15	.63	3	.022	3	7.81e-3	3	NC	5	NC	1
90			min	0	1	-.811	2	-.007	2	-1.022e-2	2	331.217	2	NC	1
91		8	max	0	15	.633	3	.023	3	8.43e-3	3	NC	5	NC	1
92			min	0	1	-.797	2	-.012	2	-1.1e-2	2	342.161	2	NC	1
93		9	max	0	15	.628	3	.024	3	9.051e-3	3	NC	5	NC	1
94			min	0	1	-.775	2	-.016	2	-1.177e-2	2	360.085	2	9548.801	3
95		10	max	0	1	.624	3	.024	3	9.671e-3	3	NC	5	NC	1
96			min	0	1	-.763	2	-.018	2	-1.255e-2	2	370.767	2	9413.766	3
97		11	max	0	1	.628	3	.024	3	9.051e-3	3	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-.775	2	-.016	2	-1.177e-2	2	360.085	2	9548.801	3
99		12	max	0	1	.633	3	.023	3	8.43e-3	3	NC	5	NC	1
100			min	0	15	-.797	2	-.012	2	-1.1e-2	2	342.161	2	NC	1
101		13	max	0	1	.63	3	.022	3	7.81e-3	3	NC	5	NC	1
102			min	0	15	-.811	2	-.007	2	-1.022e-2	2	331.217	2	NC	1
103		14	max	0	1	.613	3	.02	3	7.189e-3	3	NC	5	NC	2
104			min	0	15	-.806	2	-.005	10	-9.449e-3	2	335.25	2	7341.142	1
105		15	max	0	1	.579	3	.021	1	6.569e-3	3	NC	5	NC	2
106			min	0	15	-.772	2	-.003	10	-8.675e-3	2	362.302	2	6927.857	1
107		16	max	0	1	.526	3	.017	1	5.949e-3	3	NC	5	NC	2
108			min	0	15	-.708	2	-.003	10	-7.9e-3	2	429.57	2	8185.068	1
109		17	max	0	1	.456	3	.012	3	5.328e-3	3	NC	5	NC	1
110			min	0	15	-.612	2	-.003	10	-7.125e-3	2	590.475	2	NC	1
111		18	max	0	1	.371	3	.009	3	4.708e-3	3	NC	4	NC	1
112			min	0	15	-.493	2	-.004	2	-6.35e-3	2	1117.52	2	NC	1
113		19	max	0	1	.279	3	.008	3	4.088e-3	3	NC	1	NC	1
114			min	0	15	-.358	2	-.006	2	-5.575e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.103	2	.007	3	7.685e-3	3	NC	1	NC	1
116			min	0	1	-.094	3	-.005	2	-8.124e-3	2	NC	1	NC	1
117		2	max	0	15	.042	2	.009	3	8.314e-3	3	NC	4	NC	1
118			min	0	1	-.071	3	-.003	2	-8.428e-3	2	2476.606	2	NC	1
119		3	max	0	15	.009	9	.015	1	8.944e-3	3	NC	4	NC	2
120			min	0	1	-.054	3	-.002	10	-8.732e-3	2	1382.007	2	9582.223	1
121		4	max	0	15	.007	9	.022	1	9.573e-3	3	NC	4	NC	2
122			min	0	1	-.048	3	-.002	10	-9.037e-3	2	1106.991	2	6583.971	1
123		5	max	0	15	.007	9	.025	1	1.02e-2	3	NC	4	NC	2
124			min	0	1	-.054	3	-.002	10	-9.341e-3	2	1089.992	2	5800.497	1
125		6	max	0	15	.011	9	.023	1	1.083e-2	3	NC	4	NC	2
126			min	0	1	-.072	3	-.003	10	-9.645e-3	2	1297.601	2	6272.516	1
127		7	max	0	15	.035	1	.02	3	1.146e-2	3	NC	3	NC	2
128			min	0	1	-.098	3	-.005	10	-9.95e-3	2	2007.078	2	8647.21	1
129		8	max	0	15	.078	2	.021	3	1.209e-2	3	NC	1	NC	1
130			min	0	1	-.129	3	-.01	2	-1.025e-2	2	4402.048	3	NC	1
131		9	max	0	15	.122	2	.021	3	1.272e-2	3	NC	4	NC	1
132			min	0	1	-.154	3	-.015	2	-1.056e-2	2	2499.053	3	NC	1
133		10	max	0	1	.142	2	.021	3	1.335e-2	3	NC	4	NC	1
134			min	0	1	-.166	3	-.017	2	-1.086e-2	2	2098.813	3	NC	1
135		11	max	0	1	.122	2	.021	3	1.272e-2	3	NC	4	NC	1
136			min	0	15	-.154	3	-.015	2	-1.056e-2	2	2499.053	3	NC	1
137		12	max	0	1	.078	2	.021	3	1.209e-2	3	NC	1	NC	1
138			min	0	15	-.129	3	-.01	2	-1.025e-2	2	4402.048	3	NC	1
139		13	max	0	1	.035	1	.02	3	1.146e-2	3	NC	3	NC	2
140			min	0	15	-.098	3	-.005	10	-9.95e-3	2	2007.078	2	8647.21	1
141		14	max	0	1	.011	9	.023	1	1.083e-2	3	NC	4	NC	2
142			min	0	15	-.072	3	-.003	10	-9.645e-3	2	1297.601	2	6272.516	1
143		15	max	0	1	.007	9	.025	1	1.02e-2	3	NC	4	NC	2
144			min	0	15	-.054	3	-.002	10	-9.341e-3	2	1089.992	2	5800.497	1
145		16	max	0	1	.007	9	.022	1	9.573e-3	3	NC	4	NC	2
146			min	0	15	-.048	3	-.002	10	-9.037e-3	2	1106.991	2	6583.971	1
147		17	max	0	1	.009	9	.015	1	8.944e-3	3	NC	4	NC	2
148			min	0	15	-.054	3	-.002	10	-8.732e-3	2	1382.007	2	9582.223	1
149		18	max	0	1	.042	2	.009	3	8.314e-3	3	NC	4	NC	1
150			min	0	15	-.071	3	-.003	2	-8.428e-3	2	2476.606	2	NC	1
151		19	max	0	1	.103	2	.007	3	7.685e-3	3	NC	1	NC	1
152			min	0	15	-.094	3	-.005	2	-8.124e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.011	2	.003	1	-4.537e-6	15	NC	1	NC	1
154			min	-.01	3	-.016	3	0	15	-7.671e-5	1	7301.202	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.007	2	.009	2	.003	1	-4.307e-6	15	NC	1	NC	1
156			min	-.01	3	-.015	3	0	15	-7.282e-5	1	8450.626	2	NC	1
157		3	max	.006	2	.008	2	.002	1	-4.077e-6	15	NC	1	NC	1
158			min	-.009	3	-.015	3	0	15	-6.893e-5	1	NC	1	NC	1
159		4	max	.006	2	.006	2	.002	1	-3.848e-6	15	NC	1	NC	1
160			min	-.009	3	-.014	3	0	15	-6.504e-5	1	NC	1	NC	1
161		5	max	.005	2	.005	2	.002	1	-3.618e-6	15	NC	1	NC	1
162			min	-.008	3	-.014	3	0	15	-6.115e-5	1	NC	1	NC	1
163		6	max	.005	2	.004	2	.002	1	-3.388e-6	15	NC	1	NC	1
164			min	-.007	3	-.013	3	0	15	-5.726e-5	1	NC	1	NC	1
165		7	max	.005	2	.003	2	.002	1	-3.159e-6	15	NC	1	NC	1
166			min	-.007	3	-.013	3	0	15	-5.337e-5	1	NC	1	NC	1
167		8	max	.004	2	.002	2	.001	1	-2.929e-6	15	NC	1	NC	1
168			min	-.006	3	-.012	3	0	15	-4.949e-5	1	NC	1	NC	1
169		9	max	.004	2	0	2	.001	1	-2.699e-6	15	NC	1	NC	1
170			min	-.006	3	-.011	3	0	15	-4.56e-5	1	NC	1	NC	1
171		10	max	.004	2	0	2	0	1	-2.47e-6	15	NC	1	NC	1
172			min	-.005	3	-.01	3	0	15	-4.171e-5	1	NC	1	NC	1
173		11	max	.003	2	0	2	0	1	-2.24e-6	15	NC	1	NC	1
174			min	-.005	3	-.009	3	0	15	-3.782e-5	1	NC	1	NC	1
175		12	max	.003	2	-.001	2	0	1	-2.01e-6	15	NC	1	NC	1
176			min	-.004	3	-.008	3	0	15	-3.393e-5	1	NC	1	NC	1
177		13	max	.002	2	-.001	15	0	1	-1.781e-6	15	NC	1	NC	1
178			min	-.003	3	-.007	3	0	15	-3.004e-5	1	NC	1	NC	1
179		14	max	.002	2	-.001	15	0	1	-1.551e-6	15	NC	1	NC	1
180			min	-.003	3	-.006	3	0	15	-2.615e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-1.321e-6	15	NC	1	NC	1
182			min	-.002	3	-.005	3	0	15	-2.226e-5	1	NC	1	NC	1
183		16	max	.001	2	0	15	0	1	-1.092e-6	15	NC	1	NC	1
184			min	-.002	3	-.004	3	0	15	-1.837e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-8.621e-7	15	NC	1	NC	1
186			min	-.001	3	-.003	3	0	15	-1.448e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-6.325e-7	15	NC	1	NC	1
188			min	0	3	-.001	4	0	15	-1.06e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-4.028e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-6.706e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	1.574e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	9.493e-8	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	8.988e-6	1	NC	1	NC	1
194			min	0	2	-.002	4	0	1	5.23e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	10	1.64e-5	1	NC	1	NC	1
196			min	0	2	-.004	4	0	1	9.51e-7	15	NC	1	NC	1
197		4	max	.001	3	-.001	15	0	10	2.382e-5	1	NC	1	NC	1
198			min	-.001	2	-.006	4	0	1	1.379e-6	15	NC	1	NC	1
199		5	max	.002	3	-.002	15	0	10	3.123e-5	1	NC	1	NC	1
200			min	-.002	2	-.008	4	0	1	1.807e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	10	3.864e-5	1	NC	1	NC	1
202			min	-.002	2	-.01	4	0	1	2.235e-6	15	9260.529	4	NC	1
203		7	max	.003	3	-.003	15	0	2	4.606e-5	1	NC	1	NC	1
204			min	-.003	2	-.011	4	0	3	2.663e-6	15	8007.63	4	NC	1
205		8	max	.003	3	-.003	15	0	1	5.347e-5	1	NC	2	NC	1
206			min	-.003	2	-.013	4	0	3	3.091e-6	15	7236.569	4	NC	1
207		9	max	.004	3	-.003	15	0	1	6.089e-5	1	NC	5	NC	1
208			min	-.003	2	-.013	4	0	12	3.519e-6	15	6786.654	4	NC	1
209		10	max	.004	3	-.003	15	0	1	6.83e-5	1	NC	5	NC	1
210			min	-.004	2	-.014	4	0	15	3.947e-6	15	6580.512	4	NC	1
211		11	max	.005	3	-.003	15	0	1	7.571e-5	1	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.014	4	0	15	4.375e-6	15	6588.041	4	NC	1
213		max	.005	3	-.003	15	0	1	8.313e-5	1	NC	5	NC	1
214		min	-.005	2	-.014	4	0	15	4.803e-6	15	6814.857	4	NC	1
215		max	.006	3	-.003	15	0	1	9.054e-5	1	NC	2	NC	1
216		min	-.005	2	-.013	4	0	15	5.231e-6	15	7305.948	4	NC	1
217		max	.006	3	-.003	15	.001	1	9.796e-5	1	NC	1	NC	1
218		min	-.005	2	-.011	4	0	15	5.66e-6	15	8168.449	4	NC	1
219		max	.006	3	-.002	15	.001	1	1.054e-4	1	NC	1	NC	1
220		min	-.006	2	-.01	4	0	15	6.088e-6	15	9637.746	4	NC	1
221		max	.007	3	-.002	15	.002	1	1.128e-4	1	NC	1	NC	1
222		min	-.006	2	-.008	4	0	15	6.516e-6	15	NC	1	NC	1
223		max	.007	3	-.001	15	.002	1	1.202e-4	1	NC	1	NC	1
224		min	-.007	2	-.006	3	0	15	6.944e-6	15	NC	1	NC	1
225		max	.008	3	0	15	.002	1	1.276e-4	1	NC	1	NC	1
226		min	-.007	2	-.004	3	0	15	7.372e-6	15	NC	1	NC	1
227		max	.008	3	0	2	.003	1	1.35e-4	1	NC	1	NC	1
228		min	-.008	2	-.003	3	0	15	7.8e-6	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	5.573e-5	1	NC	1	NC	2
230		min	0	15	-.009	3	-.003	1	3.246e-6	15	NC	1	8723.374	1
231		max	.001	1	.007	2	0	15	5.573e-5	1	NC	1	NC	2
232		min	0	15	-.008	3	-.003	1	3.246e-6	15	NC	1	9450.454	1
233		max	.001	1	.007	2	0	15	5.573e-5	1	NC	1	NC	1
234		min	0	15	-.008	3	-.002	1	3.246e-6	15	NC	1	NC	1
235		max	.001	1	.006	2	0	15	5.573e-5	1	NC	1	NC	1
236		min	0	15	-.007	3	-.002	1	3.246e-6	15	NC	1	NC	1
237		max	.001	1	.006	2	0	15	5.573e-5	1	NC	1	NC	1
238		min	0	15	-.007	3	-.002	1	3.246e-6	15	NC	1	NC	1
239		max	.001	1	.005	2	0	15	5.573e-5	1	NC	1	NC	1
240		min	0	15	-.006	3	-.002	1	3.246e-6	15	NC	1	NC	1
241		max	.001	1	.005	2	0	15	5.573e-5	1	NC	1	NC	1
242		min	0	15	-.006	3	-.002	1	3.246e-6	15	NC	1	NC	1
243		max	0	1	.005	2	0	15	5.573e-5	1	NC	1	NC	1
244		min	0	15	-.005	3	-.001	1	3.246e-6	15	NC	1	NC	1
245		max	0	1	.004	2	0	15	5.573e-5	1	NC	1	NC	1
246		min	0	15	-.005	3	-.001	1	3.246e-6	15	NC	1	NC	1
247		max	0	1	.004	2	0	15	5.573e-5	1	NC	1	NC	1
248		min	0	15	-.004	3	0	1	3.246e-6	15	NC	1	NC	1
249		max	0	1	.003	2	0	15	5.573e-5	1	NC	1	NC	1
250		min	0	15	-.004	3	0	1	3.246e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	5.573e-5	1	NC	1	NC	1
252		min	0	15	-.003	3	0	1	3.246e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	5.573e-5	1	NC	1	NC	1
254		min	0	15	-.003	3	0	1	3.246e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	5.573e-5	1	NC	1	NC	1
256		min	0	15	-.002	3	0	1	3.246e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	5.573e-5	1	NC	1	NC	1
258		min	0	15	-.002	3	0	1	3.246e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	5.573e-5	1	NC	1	NC	1
260		min	0	15	-.001	3	0	1	3.246e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	5.573e-5	1	NC	1	NC	1
262		min	0	15	0	3	0	1	3.246e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	5.573e-5	1	NC	1	NC	1
264		min	0	15	0	3	0	1	3.246e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	5.573e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	3.246e-6	15	NC	1	NC	1
267	M6	max	.021	2	.033	2	0	1	0	1	NC	4	NC	1
268		min	-.032	3	-.047	3	0	1	0	1	1628.432	3	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.02	2	.03	2	0	1	0	1	NC	4	NC	1
270		min	-.03	3	-.045	3	0	1	0	1	1723.556	3	NC	1
271	3	max	.019	2	.027	2	0	1	0	1	NC	4	NC	1
272		min	-.028	3	-.042	3	0	1	0	1	1830.633	3	NC	1
273	4	max	.017	2	.024	2	0	1	0	1	NC	4	NC	1
274		min	-.026	3	-.039	3	0	1	0	1	1952.204	3	NC	1
275	5	max	.016	2	.022	2	0	1	0	1	NC	4	NC	1
276		min	-.025	3	-.037	3	0	1	0	1	2091.534	3	NC	1
277	6	max	.015	2	.019	2	0	1	0	1	NC	4	NC	1
278		min	-.023	3	-.034	3	0	1	0	1	2252.893	3	NC	1
279	7	max	.014	2	.016	2	0	1	0	1	NC	1	NC	1
280		min	-.021	3	-.032	3	0	1	0	1	2441.977	3	NC	1
281	8	max	.013	2	.014	2	0	1	0	1	NC	1	NC	1
282		min	-.019	3	-.029	3	0	1	0	1	2666.552	3	NC	1
283	9	max	.012	2	.011	2	0	1	0	1	NC	1	NC	1
284		min	-.018	3	-.026	3	0	1	0	1	2937.5	3	NC	1
285	10	max	.01	2	.009	2	0	1	0	1	NC	1	NC	1
286		min	-.016	3	-.024	3	0	1	0	1	3270.543	3	NC	1
287	11	max	.009	2	.007	2	0	1	0	1	NC	1	NC	1
288		min	-.014	3	-.021	3	0	1	0	1	3689.276	3	NC	1
289	12	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.012	3	-.018	3	0	1	0	1	4230.796	3	NC	1
291	13	max	.007	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.011	3	-.016	3	0	1	0	1	4956.956	3	NC	1
293	14	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.009	3	-.013	3	0	1	0	1	5979.131	3	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.007	3	-.01	3	0	1	0	1	7520.136	3	NC	1
297	16	max	.003	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	0	15	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.007	2	-.012	3	0	1	0	1	8887.592	3	NC	1
317	7	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.014	3	0	1	0	1	7939.913	3	NC	1
319	8	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7298.589	4	NC	1
321	9	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.011	2	-.016	3	0	1	0	1	6841.345	4	NC	1
323	10	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.012	2	-.016	3	0	1	0	1	6630.693	4	NC	1
325	11	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.017	3	0	1	0	1	6635.859	4	NC	1
327		12	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.015	2	-.016	3	0	1	0	1	6862.196	4	NC	1
329		13	max	.016	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.016	2	-.016	3	0	1	0	1	7354.775	4	NC	1
331		14	max	.018	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.017	2	-.015	3	0	1	0	1	8221.243	4	NC	1
333		15	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.019	2	-.014	3	0	1	0	1	9698.301	4	NC	1
335		16	max	.02	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.02	2	-.012	3	0	1	0	1	NC	1	NC	1
337		17	max	.022	3	0	2	0	1	0	1	NC	1	NC	1
338			min	-.021	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.023	3	0	2	0	1	0	1	NC	1	NC	1
340			min	-.022	2	-.009	3	0	1	0	1	NC	1	NC	1
341		19	max	.024	3	.002	2	0	1	0	1	NC	1	NC	1
342			min	-.024	2	-.007	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.004	2	.024	2	0	1	0	1	NC	1	NC	1
344			min	0	15	-.025	3	0	1	0	1	NC	1	NC	1
345		2	max	.004	2	.022	2	0	1	0	1	NC	1	NC	1
346			min	0	15	-.024	3	0	1	0	1	NC	1	NC	1
347		3	max	.004	2	.021	2	0	1	0	1	NC	1	NC	1
348			min	0	15	-.023	3	0	1	0	1	NC	1	NC	1
349		4	max	.004	2	.02	2	0	1	0	1	NC	1	NC	1
350			min	0	15	-.021	3	0	1	0	1	NC	1	NC	1
351		5	max	.003	2	.018	2	0	1	0	1	NC	1	NC	1
352			min	0	15	-.02	3	0	1	0	1	NC	1	NC	1
353		6	max	.003	2	.017	2	0	1	0	1	NC	1	NC	1
354			min	0	15	-.018	3	0	1	0	1	NC	1	NC	1
355		7	max	.003	2	.016	2	0	1	0	1	NC	1	NC	1
356			min	0	15	-.017	3	0	1	0	1	NC	1	NC	1
357		8	max	.003	2	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	15	-.016	3	0	1	0	1	NC	1	NC	1
359		9	max	.002	2	.013	2	0	1	0	1	NC	1	NC	1
360			min	0	15	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.002	2	.012	2	0	1	0	1	NC	1	NC	1
362			min	0	15	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.002	2	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	15	-.011	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	2	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	15	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.001	2	.008	2	0	1	0	1	NC	1	NC	1
368			min	0	15	-.008	3	0	1	0	1	NC	1	NC	1
369		14	max	.001	2	.007	2	0	1	0	1	NC	1	NC	1
370			min	0	15	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	0	2	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	15	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	0	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	15	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	15	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	15	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.011	2	0	15	7.671e-5	1	NC	1	NC	1
382			min	-.01	3	-.016	3	-.003	1	4.537e-6	15	7301.202	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	2	.009	2	0	15	7.282e-5	1	NC	1	NC	1
384		min	-.01	3	-.015	3	-.003	1	4.307e-6	15	8450.626	2	NC	1
385	3	max	.006	2	.008	2	0	15	6.893e-5	1	NC	1	NC	1
386		min	-.009	3	-.015	3	-.002	1	4.077e-6	15	NC	1	NC	1
387	4	max	.006	2	.006	2	0	15	6.504e-5	1	NC	1	NC	1
388		min	-.009	3	-.014	3	-.002	1	3.848e-6	15	NC	1	NC	1
389	5	max	.005	2	.005	2	0	15	6.115e-5	1	NC	1	NC	1
390		min	-.008	3	-.014	3	-.002	1	3.618e-6	15	NC	1	NC	1
391	6	max	.005	2	.004	2	0	15	5.726e-5	1	NC	1	NC	1
392		min	-.007	3	-.013	3	-.002	1	3.388e-6	15	NC	1	NC	1
393	7	max	.005	2	.003	2	0	15	5.337e-5	1	NC	1	NC	1
394		min	-.007	3	-.013	3	-.002	1	3.159e-6	15	NC	1	NC	1
395	8	max	.004	2	.002	2	0	15	4.949e-5	1	NC	1	NC	1
396		min	-.006	3	-.012	3	-.001	1	2.929e-6	15	NC	1	NC	1
397	9	max	.004	2	0	2	0	15	4.56e-5	1	NC	1	NC	1
398		min	-.006	3	-.011	3	-.001	1	2.699e-6	15	NC	1	NC	1
399	10	max	.004	2	0	2	0	15	4.171e-5	1	NC	1	NC	1
400		min	-.005	3	-.01	3	0	1	2.47e-6	15	NC	1	NC	1
401	11	max	.003	2	0	2	0	15	3.782e-5	1	NC	1	NC	1
402		min	-.005	3	-.009	3	0	1	2.24e-6	15	NC	1	NC	1
403	12	max	.003	2	-.001	2	0	15	3.393e-5	1	NC	1	NC	1
404		min	-.004	3	-.008	3	0	1	2.01e-6	15	NC	1	NC	1
405	13	max	.002	2	-.001	15	0	15	3.004e-5	1	NC	1	NC	1
406		min	-.003	3	-.007	3	0	1	1.781e-6	15	NC	1	NC	1
407	14	max	.002	2	-.001	15	0	15	2.615e-5	1	NC	1	NC	1
408		min	-.003	3	-.006	3	0	1	1.551e-6	15	NC	1	NC	1
409	15	max	.002	2	-.001	15	0	15	2.226e-5	1	NC	1	NC	1
410		min	-.002	3	-.005	3	0	1	1.321e-6	15	NC	1	NC	1
411	16	max	.001	2	0	15	0	15	1.837e-5	1	NC	1	NC	1
412		min	-.002	3	-.004	3	0	1	1.092e-6	15	NC	1	NC	1
413	17	max	0	2	0	15	0	15	1.448e-5	1	NC	1	NC	1
414		min	-.001	3	-.003	3	0	1	8.621e-7	15	NC	1	NC	1
415	18	max	0	2	0	15	0	15	1.06e-5	1	NC	1	NC	1
416		min	0	3	-.001	4	0	1	6.325e-7	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	6.706e-6	1	NC	1	NC	1
418		min	0	1	0	1	0	1	4.028e-7	15	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-9.493e-8	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-1.574e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-5.23e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	15	-8.988e-6	1	NC	1	NC	1
423	3	max	0	3	0	15	0	1	-9.51e-7	15	NC	1	NC	1
424		min	0	2	-.004	4	0	10	-1.64e-5	1	NC	1	NC	1
425	4	max	.001	3	-.001	15	0	1	-1.379e-6	15	NC	1	NC	1
426		min	-.001	2	-.006	4	0	10	-2.382e-5	1	NC	1	NC	1
427	5	max	.002	3	-.002	15	0	1	-1.807e-6	15	NC	1	NC	1
428		min	-.002	2	-.008	4	0	10	-3.123e-5	1	NC	1	NC	1
429	6	max	.002	3	-.002	15	0	1	-2.235e-6	15	NC	1	NC	1
430		min	-.002	2	-.01	4	0	10	-3.864e-5	1	9260.529	4	NC	1
431	7	max	.003	3	-.003	15	0	3	-2.663e-6	15	NC	1	NC	1
432		min	-.003	2	-.011	4	0	2	-4.606e-5	1	8007.63	4	NC	1
433	8	max	.003	3	-.003	15	0	3	-3.091e-6	15	NC	2	NC	1
434		min	-.003	2	-.013	4	0	1	-5.347e-5	1	7236.569	4	NC	1
435	9	max	.004	3	-.003	15	0	12	-3.519e-6	15	NC	5	NC	1
436		min	-.003	2	-.013	4	0	1	-6.089e-5	1	6786.654	4	NC	1
437	10	max	.004	3	-.003	15	0	15	-3.947e-6	15	NC	5	NC	1
438		min	-.004	2	-.014	4	0	1	-6.83e-5	1	6580.512	4	NC	1
439	11	max	.005	3	-.003	15	0	15	-4.375e-6	15	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440			min	-.004	2	-.014	4	0	1	-7.571e-5	1	6588.041	4	NC	1
441		12	max	.005	3	-.003	15	0	15	-4.803e-6	15	NC	5	NC	1
442			min	-.005	2	-.014	4	0	1	-8.313e-5	1	6814.857	4	NC	1
443		13	max	.006	3	-.003	15	0	15	-5.231e-6	15	NC	2	NC	1
444			min	-.005	2	-.013	4	0	1	-9.054e-5	1	7305.948	4	NC	1
445		14	max	.006	3	-.003	15	0	15	-5.66e-6	15	NC	1	NC	1
446			min	-.005	2	-.011	4	-.001	1	-9.796e-5	1	8168.449	4	NC	1
447		15	max	.006	3	-.002	15	0	15	-6.088e-6	15	NC	1	NC	1
448			min	-.006	2	-.01	4	-.001	1	-1.054e-4	1	9637.746	4	NC	1
449		16	max	.007	3	-.002	15	0	15	-6.516e-6	15	NC	1	NC	1
450			min	-.006	2	-.008	4	-.002	1	-1.128e-4	1	NC	1	NC	1
451		17	max	.007	3	-.001	15	0	15	-6.944e-6	15	NC	1	NC	1
452			min	-.007	2	-.006	3	-.002	1	-1.202e-4	1	NC	1	NC	1
453		18	max	.008	3	0	15	0	15	-7.372e-6	15	NC	1	NC	1
454			min	-.007	2	-.004	3	-.002	1	-1.276e-4	1	NC	1	NC	1
455		19	max	.008	3	0	2	0	15	-7.8e-6	15	NC	1	NC	1
456			min	-.008	2	-.003	3	-.003	1	-1.35e-4	1	NC	1	NC	1
457	M12	1	max	.002	1	.007	2	.003	1	-3.246e-6	15	NC	1	NC	2
458			min	0	15	-.009	3	0	15	-5.573e-5	1	NC	1	8723.374	1
459		2	max	.001	1	.007	2	.003	1	-3.246e-6	15	NC	1	NC	2
460			min	0	15	-.008	3	0	15	-5.573e-5	1	NC	1	9450.454	1
461		3	max	.001	1	.007	2	.002	1	-3.246e-6	15	NC	1	NC	1
462			min	0	15	-.008	3	0	15	-5.573e-5	1	NC	1	NC	1
463		4	max	.001	1	.006	2	.002	1	-3.246e-6	15	NC	1	NC	1
464			min	0	15	-.007	3	0	15	-5.573e-5	1	NC	1	NC	1
465		5	max	.001	1	.006	2	.002	1	-3.246e-6	15	NC	1	NC	1
466			min	0	15	-.007	3	0	15	-5.573e-5	1	NC	1	NC	1
467		6	max	.001	1	.005	2	.002	1	-3.246e-6	15	NC	1	NC	1
468			min	0	15	-.006	3	0	15	-5.573e-5	1	NC	1	NC	1
469		7	max	.001	1	.005	2	.002	1	-3.246e-6	15	NC	1	NC	1
470			min	0	15	-.006	3	0	15	-5.573e-5	1	NC	1	NC	1
471		8	max	0	1	.005	2	.001	1	-3.246e-6	15	NC	1	NC	1
472			min	0	15	-.005	3	0	15	-5.573e-5	1	NC	1	NC	1
473		9	max	0	1	.004	2	.001	1	-3.246e-6	15	NC	1	NC	1
474			min	0	15	-.005	3	0	15	-5.573e-5	1	NC	1	NC	1
475		10	max	0	1	.004	2	0	1	-3.246e-6	15	NC	1	NC	1
476			min	0	15	-.004	3	0	15	-5.573e-5	1	NC	1	NC	1
477		11	max	0	1	.003	2	0	1	-3.246e-6	15	NC	1	NC	1
478			min	0	15	-.004	3	0	15	-5.573e-5	1	NC	1	NC	1
479		12	max	0	1	.003	2	0	1	-3.246e-6	15	NC	1	NC	1
480			min	0	15	-.003	3	0	15	-5.573e-5	1	NC	1	NC	1
481		13	max	0	1	.002	2	0	1	-3.246e-6	15	NC	1	NC	1
482			min	0	15	-.003	3	0	15	-5.573e-5	1	NC	1	NC	1
483		14	max	0	1	.002	2	0	1	-3.246e-6	15	NC	1	NC	1
484			min	0	15	-.002	3	0	15	-5.573e-5	1	NC	1	NC	1
485		15	max	0	1	.002	2	0	1	-3.246e-6	15	NC	1	NC	1
486			min	0	15	-.002	3	0	15	-5.573e-5	1	NC	1	NC	1
487		16	max	0	1	.001	2	0	1	-3.246e-6	15	NC	1	NC	1
488			min	0	15	-.001	3	0	15	-5.573e-5	1	NC	1	NC	1
489		17	max	0	1	0	2	0	1	-3.246e-6	15	NC	1	NC	1
490			min	0	15	0	3	0	15	-5.573e-5	1	NC	1	NC	1
491		18	max	0	1	0	2	0	1	-3.246e-6	15	NC	1	NC	1
492			min	0	15	0	3	0	15	-5.573e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-3.246e-6	15	NC	1	NC	1
494			min	0	1	0	1	0	1	-5.573e-5	1	NC	1	NC	1
495	M1	1	max	.01	3	.116	2	0	1	4.031e-3	2	NC	1	NC	1
496			min	-.007	2	-.035	3	0	15	-1.092e-2	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.01	3	.054	2	0	15	1.978e-3	2	NC	4	NC	1
498		min	-.007	2	-.013	3	-.002	1	-5.411e-3	3	1846.93	2	NC	1
499	3	max	.01	3	.017	3	0	15	2.887e-5	2	NC	5	NC	1
500		min	-.007	2	-.012	2	-.003	1	-8.404e-5	3	897.337	2	NC	1
501	4	max	.01	3	.06	3	0	15	2.51e-3	2	NC	5	NC	1
502		min	-.006	2	-.085	2	-.003	1	-2.779e-3	3	573.229	2	NC	1
503	5	max	.01	3	.112	3	0	15	4.99e-3	2	NC	5	NC	1
504		min	-.006	2	-.16	2	-.002	1	-5.474e-3	3	417.969	2	NC	1
505	6	max	.01	3	.167	3	0	15	7.471e-3	2	NC	5	NC	1
506		min	-.006	2	-.231	2	0	1	-8.169e-3	3	331.837	2	NC	1
507	7	max	.009	3	.219	3	0	1	9.951e-3	2	NC	15	NC	1
508		min	-.006	2	-.294	2	0	3	-1.086e-2	3	280.689	2	NC	1
509	8	max	.009	3	.262	3	0	1	1.243e-2	2	NC	15	NC	1
510		min	-.006	2	-.344	2	0	15	-1.356e-2	3	250.299	2	NC	1
511	9	max	.009	3	.289	3	0	15	1.399e-2	2	NC	15	NC	1
512		min	-.006	2	-.376	2	0	1	-1.401e-2	3	234.427	2	NC	1
513	10	max	.009	3	.299	3	0	1	1.494e-2	2	NC	15	NC	1
514		min	-.006	2	-.386	2	0	10	-1.296e-2	3	229.807	2	NC	1
515	11	max	.009	3	.291	3	0	1	1.588e-2	2	NC	15	NC	1
516		min	-.006	2	-.375	2	0	15	-1.192e-2	3	235.375	2	NC	1
517	12	max	.008	3	.267	3	0	15	1.525e-2	2	NC	15	NC	1
518		min	-.006	2	-.342	2	0	1	-1.046e-2	3	253.097	2	NC	1
519	13	max	.008	3	.228	3	0	10	1.222e-2	2	NC	15	NC	1
520		min	-.006	2	-.289	2	0	1	-8.371e-3	3	287.32	2	NC	1
521	14	max	.008	3	.178	3	0	1	9.199e-3	2	NC	5	NC	1
522		min	-.005	2	-.223	2	0	15	-6.282e-3	3	345.703	2	NC	1
523	15	max	.008	3	.122	3	.002	1	6.175e-3	2	NC	5	NC	1
524		min	-.005	2	-.15	2	0	15	-4.194e-3	3	445.909	2	NC	1
525	16	max	.007	3	.063	3	.003	1	3.151e-3	2	NC	5	NC	1
526		min	-.005	2	-.076	2	0	15	-2.105e-3	3	630.784	2	NC	1
527	17	max	.007	3	.006	3	.003	1	2.071e-4	1	NC	5	NC	1
528		min	-.005	2	-.007	2	0	15	-1.612e-5	3	1024.606	2	NC	1
529	18	max	.007	3	.051	2	.002	1	3.782e-3	2	NC	4	NC	1
530		min	-.005	2	-.046	3	0	15	-1.458e-3	3	2165.864	2	NC	1
531	19	max	.007	3	.103	2	0	15	7.592e-3	2	NC	1	NC	1
532		min	-.005	2	-.094	3	0	1	-2.983e-3	3	NC	1	NC	1
533	M5	1	max	.03	.182	2	0	1	0	1	NC	1	NC	1
534		min	-.021	2	.003	15	0	1	0	1	NC	1	NC	1
535	2	max	.03	3	.08	2	0	1	0	1	NC	4	NC	1
536		min	-.021	2	.001	15	0	1	0	1	1142.05	2	NC	1
537	3	max	.03	3	.05	3	0	1	0	1	NC	5	NC	1
538		min	-.021	2	-.036	2	0	1	0	1	534.396	2	NC	1
539	4	max	.029	3	.129	3	0	1	0	1	NC	5	NC	1
540		min	-.021	2	-.176	2	0	1	0	1	324.719	2	NC	1
541	5	max	.028	3	.238	3	0	1	0	1	NC	15	NC	1
542		min	-.021	2	-.328	2	0	1	0	1	227.234	2	NC	1
543	6	max	.028	3	.362	3	0	1	0	1	NC	15	NC	1
544		min	-.02	2	-.481	2	0	1	0	1	174.898	2	NC	1
545	7	max	.027	3	.483	3	0	1	0	1	8436.893	15	NC	1
546		min	-.02	2	-.619	2	0	1	0	1	144.651	2	NC	1
547	8	max	.027	3	.585	3	0	1	0	1	7398.832	15	NC	1
548		min	-.02	2	-.731	2	0	1	0	1	127.053	2	NC	1
549	9	max	.026	3	.649	3	0	1	0	1	6868.097	15	NC	1
550		min	-.019	2	-.801	2	0	1	0	1	118.023	2	NC	1
551	10	max	.025	3	.672	3	0	1	0	1	6708.587	15	NC	1
552		min	-.019	2	-.825	2	0	1	0	1	115.402	2	NC	1
553	11	max	.025	3	.654	3	0	1	0	1	6868.989	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.019	2	-.801	2	0	1	0	1	118.562	2	NC	1
555	12	max	.024	3	.596	3	0	1	0	1	7400.865	15	NC	1
556		min	-.018	2	-.727	2	0	1	0	1	128.871	2	NC	1
557	13	max	.023	3	.504	3	0	1	0	1	8440.802	15	NC	1
558		min	-.018	2	-.607	2	0	1	0	1	149.486	2	NC	1
559	14	max	.023	3	.389	3	0	1	0	1	NC	15	NC	1
560		min	-.018	2	-.459	2	0	1	0	1	186.05	2	NC	1
561	15	max	.022	3	.262	3	0	1	0	1	NC	15	NC	1
562		min	-.017	2	-.301	2	0	1	0	1	252.196	2	NC	1
563	16	max	.022	3	.134	3	0	1	0	1	NC	5	NC	1
564		min	-.017	2	-.148	2	0	1	0	1	383.146	2	NC	1
565	17	max	.021	3	.017	3	0	1	0	1	NC	5	NC	1
566		min	-.017	2	-.02	2	0	1	0	1	685.252	2	NC	1
567	18	max	.021	3	.072	2	0	1	0	1	NC	4	NC	1
568		min	-.017	2	-.08	3	0	1	0	1	1497.936	3	NC	1
569	19	max	.021	3	.142	2	0	1	0	1	NC	1	NC	1
570		min	-.017	2	-.166	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.116	2	0	15	1.092e-2	3	NC	1	NC	1
572		min	-.007	2	-.035	3	0	1	-4.031e-3	2	NC	1	NC	1
573	2	max	.01	3	.054	2	.002	1	5.411e-3	3	NC	4	NC	1
574		min	-.007	2	-.013	3	0	15	-1.978e-3	2	1846.93	2	NC	1
575	3	max	.01	3	.017	3	.003	1	8.404e-5	3	NC	5	NC	1
576		min	-.007	2	-.012	2	0	15	-2.887e-5	2	897.337	2	NC	1
577	4	max	.01	3	.06	3	.003	1	2.779e-3	3	NC	5	NC	1
578		min	-.006	2	-.085	2	0	15	-2.51e-3	2	573.229	2	NC	1
579	5	max	.01	3	.112	3	.002	1	5.474e-3	3	NC	5	NC	1
580		min	-.006	2	-.16	2	0	15	-4.99e-3	2	417.969	2	NC	1
581	6	max	.01	3	.167	3	0	1	8.169e-3	3	NC	5	NC	1
582		min	-.006	2	-.231	2	0	15	-7.471e-3	2	331.837	2	NC	1
583	7	max	.009	3	.219	3	0	3	1.086e-2	3	NC	15	NC	1
584		min	-.006	2	-.294	2	0	1	-9.951e-3	2	280.689	2	NC	1
585	8	max	.009	3	.262	3	0	15	1.356e-2	3	NC	15	NC	1
586		min	-.006	2	-.344	2	0	1	-1.243e-2	2	250.299	2	NC	1
587	9	max	.009	3	.289	3	0	1	1.401e-2	3	NC	15	NC	1
588		min	-.006	2	-.376	2	0	15	-1.399e-2	2	234.427	2	NC	1
589	10	max	.009	3	.299	3	0	10	1.296e-2	3	NC	15	NC	1
590		min	-.006	2	-.386	2	0	1	-1.494e-2	2	229.807	2	NC	1
591	11	max	.009	3	.291	3	0	15	1.192e-2	3	NC	15	NC	1
592		min	-.006	2	-.375	2	0	1	-1.588e-2	2	235.375	2	NC	1
593	12	max	.008	3	.267	3	0	1	1.046e-2	3	NC	15	NC	1
594		min	-.006	2	-.342	2	0	15	-1.525e-2	2	253.097	2	NC	1
595	13	max	.008	3	.228	3	0	1	8.371e-3	3	NC	15	NC	1
596		min	-.006	2	-.289	2	0	10	-1.222e-2	2	287.32	2	NC	1
597	14	max	.008	3	.178	3	0	15	6.282e-3	3	NC	5	NC	1
598		min	-.005	2	-.223	2	0	1	-9.199e-3	2	345.703	2	NC	1
599	15	max	.008	3	.122	3	0	15	4.194e-3	3	NC	5	NC	1
600		min	-.005	2	-.15	2	-.002	1	-6.175e-3	2	445.909	2	NC	1
601	16	max	.007	3	.063	3	0	15	2.105e-3	3	NC	5	NC	1
602		min	-.005	2	-.076	2	-.003	1	-3.151e-3	2	630.784	2	NC	1
603	17	max	.007	3	.006	3	0	15	1.612e-5	3	NC	5	NC	1
604		min	-.005	2	-.007	2	-.003	1	-2.071e-4	1	1024.606	2	NC	1
605	18	max	.007	3	.051	2	0	15	1.458e-3	3	NC	4	NC	1
606		min	-.005	2	-.046	3	-.002	1	-3.782e-3	2	2165.864	2	NC	1
607	19	max	.007	3	.103	2	0	1	2.983e-3	3	NC	1	NC	1
608		min	-.005	2	-.094	3	0	15	-7.592e-3	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1723
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
Adhesive	1723	5365	0.32	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	593	3156	0.19	Pass (Governs)	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

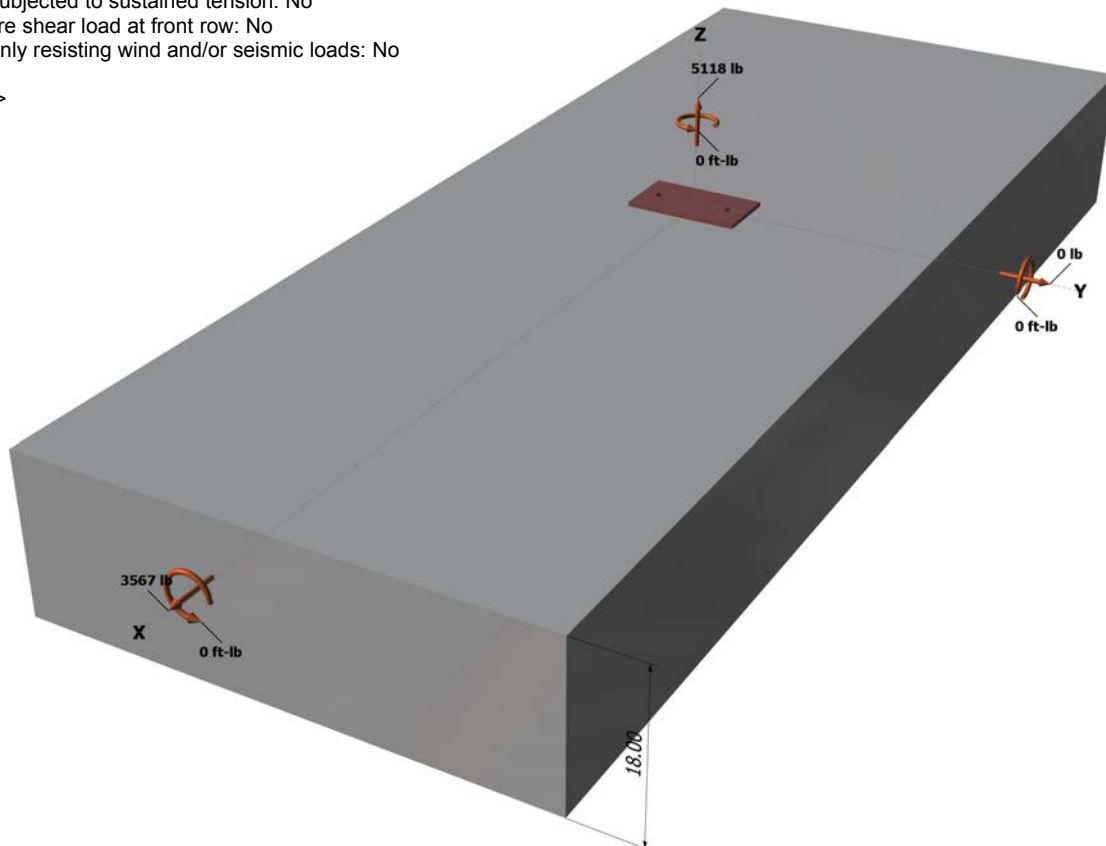
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

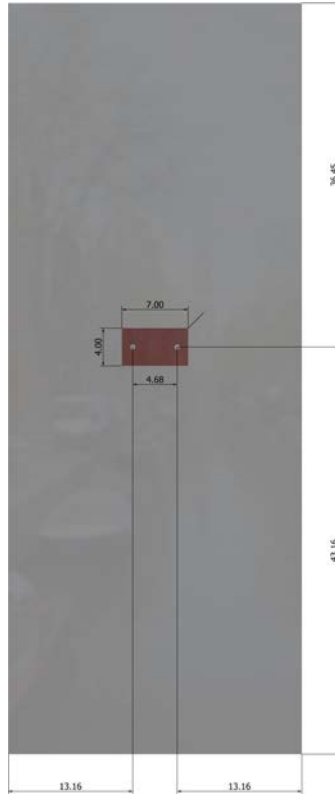
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

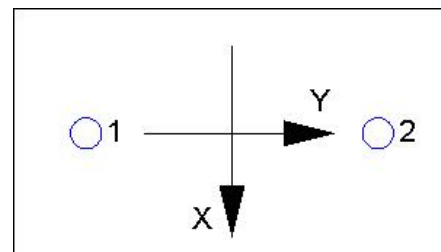
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 5118
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
Adhesive	5118	8093	0.63	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	1784	3156	0.57	Pass (Governs)	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
------------	------	------	---------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.