

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	25° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+}$ TOP =	1.100	(Pressure)
$C_{f+}$ BOTTOM =	1.700	
$C_{f-}$ TOP, OUTER PURLIN =	-2.500	
$C_{f-}$ TOP, INNER PURLIN =	-1.900	(Suction)
$C_{f-}$ BOTTOM =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	117 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.795 k-ft
$M_z$ =	0.372 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>97%</b>

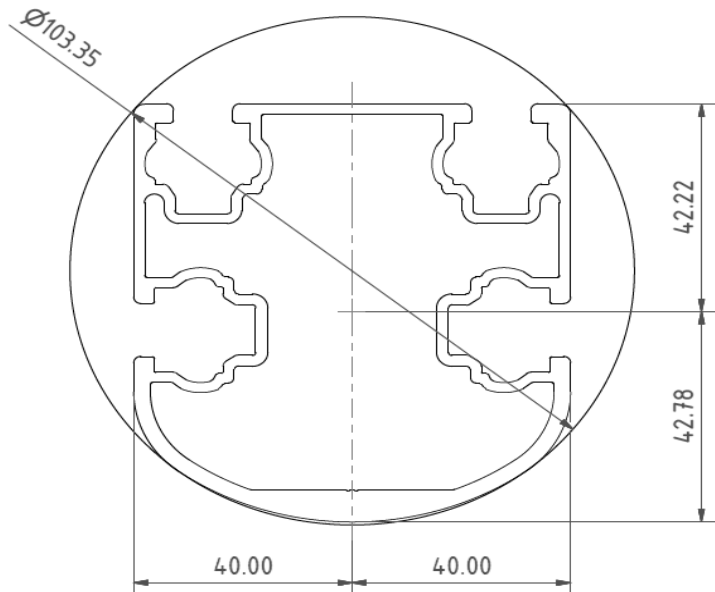


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.252 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.354 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>96%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.094 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>11%</u>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

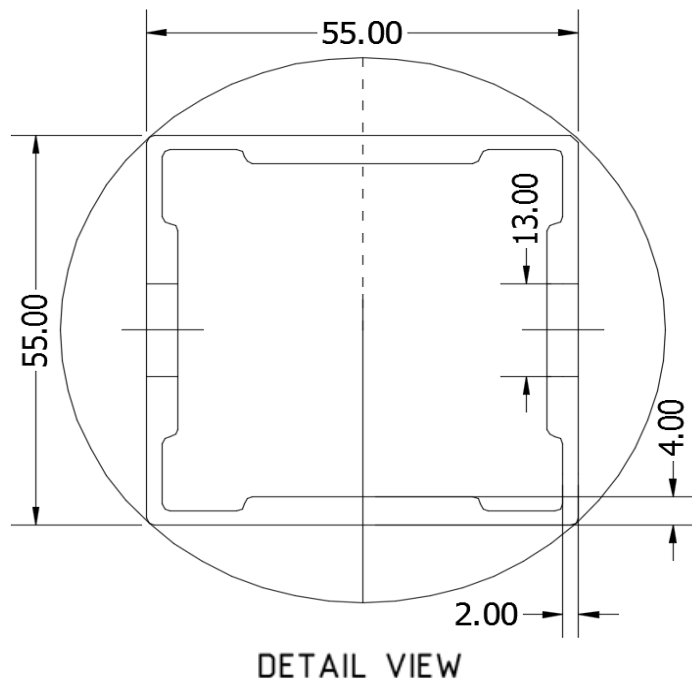
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.770 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<u>30%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	69.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.82 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.352 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.629 k
Utilization =	<u>32%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

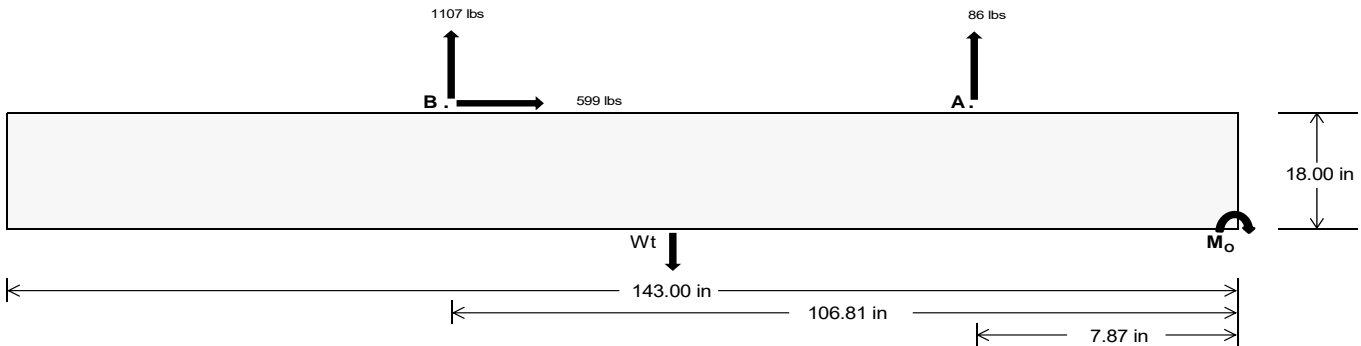
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>394.34</u>	<u>4820.59</u>	k
Compressive Load =	<u>4021.68</u>	<u>4680.48</u>	k
Lateral Load =	<u>21.46</u>	<u>2596.58</u>	k
Moment (Weak Axis) =	<u>0.04</u>	<u>0.01</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 129694.3$  in-lbs  
Resisting Force Required = 1813.91 lbs  
S.F. = 1.67  
Weight Required = 3023.18 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 598.73 lbs  
Friction = 0.4  
Weight Required = 1496.82 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 598.73 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$  7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1537 lbs	1537 lbs	1537 lbs	1537 lbs	1181 lbs	1181 lbs	1181 lbs	1181 lbs	1898 lbs	1898 lbs	1898 lbs	1898 lbs	-171 lbs	-171 lbs	-171 lbs	-171 lbs
$F_B$	1604 lbs	1604 lbs	1604 lbs	1604 lbs	1669 lbs	1669 lbs	1669 lbs	1669 lbs	2309 lbs	2309 lbs	2309 lbs	2309 lbs	-2214 lbs	-2214 lbs	-2214 lbs	-2214 lbs
$F_V$	202 lbs	202 lbs	202 lbs	202 lbs	1093 lbs	1093 lbs	1093 lbs	1093 lbs	954 lbs	954 lbs	954 lbs	954 lbs	-1197 lbs	-1197 lbs	-1197 lbs	-1197 lbs
$P_{total}$	10700 lbs	10916 lbs	11132 lbs	11348 lbs	10410 lbs	10626 lbs	10842 lbs	11058 lbs	11766 lbs	11982 lbs	12198 lbs	12414 lbs	2151 lbs	2280 lbs	2410 lbs	2539 lbs
$M$	3731 lbs-ft	3731 lbs-ft	3731 lbs-ft	3731 lbs-ft	2992 lbs-ft	2992 lbs-ft	2992 lbs-ft	2992 lbs-ft	4700 lbs-ft	4700 lbs-ft	4700 lbs-ft	4700 lbs-ft	3811 lbs-ft	3811 lbs-ft	3811 lbs-ft	3811 lbs-ft
$e$	0.35 ft	0.34 ft	0.34 ft	0.33 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.40 ft	0.39 ft	0.39 ft	0.38 ft	1.77 ft	1.67 ft	1.58 ft	1.50 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	253.8 psf	252.8 psf	251.9 psf	250.9 psf	256.2 psf	255.1 psf	254.1 psf	253.1 psf	270.5 psf	269.0 psf	267.6 psf	266.3 psf	6.7 psf	10.1 psf	13.4 psf	16.4 psf
$f_{max}$	361.9 psf	357.9 psf	354.1 psf	350.5 psf	342.8 psf	339.4 psf	336.1 psf	332.9 psf	406.6 psf	401.4 psf	396.4 psf	391.7 psf	117.1 psf	117.5 psf	117.8 psf	118.1 psf

Maximum Bearing Pressure = 407 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

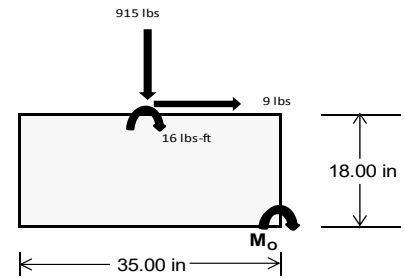
#### Overturning Check

$M_o = 1305.9 \text{ ft-lbs}$   
 Resisting Force Required = 895.50 lbs  
 S.F. = 1.67  
 Weight Required = 1492.50 lbs  
 Minimum Width = **35 in**  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	277 lbs	710 lbs	277 lbs	915 lbs	2602 lbs	915 lbs	81 lbs	208 lbs	81 lbs
$F_v$	2 lbs	0 lbs	2 lbs	9 lbs	0 lbs	9 lbs	1 lbs	0 lbs	1 lbs
$P_{total}$	9636 lbs	7560 lbs	9636 lbs	9824 lbs	7560 lbs	9824 lbs	2818 lbs	7560 lbs	2818 lbs
$M$	8 lbs-ft	0 lbs-ft	8 lbs-ft	29 lbs-ft	0 lbs-ft	29 lbs-ft	2 lbs-ft	0 lbs-ft	2 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	276.8 psf	217.5 psf	276.8 psf	281.0 psf	217.5 psf	281.0 psf	81.0 psf	217.5 psf	81.0 psf
$f_{max}$	277.7 psf	217.5 psf	277.7 psf	284.4 psf	217.5 psf	284.4 psf	81.2 psf	217.5 psf	81.2 psf



Maximum Bearing Pressure = 284 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 143in long x 24in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.519 k
Allowable Uplift =	1.214 k
Utilization =	<u>43%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.785 k
Allowable Uplift =	4.357 k
Utilization =	<u>41%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.094 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>42%</u>

#### Rear Strut

Maximum Axial Load =	3.352 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>45%</u>

#### Diagonal Strut

Maximum Axial Load =	1.854 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>25%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	56.48 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.130 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 117 \text{ in}$$

$$J = 0.432$$

$$323.677$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 27.5 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 117$$

$$J = 0.432$$

$$205.839$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 28.7$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.80 \text{ in} \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.8 \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.61471$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.80606$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 10.8205 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 10.82 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 11.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-55.176	-55.176	0	0
2	M14	Y	-55.176	-55.176	0	0
3	M15	Y	-55.176	-55.176	0	0
4	M16	Y	-55.176	-55.176	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-68.563	-68.563	0	0
2	M14	y	-68.563	-68.563	0	0
3	M15	y	-105.961	-105.961	0	0
4	M16	y	-105.961	-105.961	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	155.825	155.825	0	0
2	M14	y	118.427	118.427	0	0
3	M15	y	62.33	62.33	0	0
4	M16	y	62.33	62.33	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 25° 110mph 30psf 9.75ft 7-10 NS.r3d] Page 19



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	99.654	1	230.322	1	-.663	12	.014	1	-.007	15	.775	3
28			min	4.045	15	-263.591	3	-32.969	1	0	15	-.174	1	-.61	1
29		15	max	99.654	1	93.137	1	13.684	1	.014	1	-.007	12	.972	3
30			min	4.045	15	-100.929	3	.568	15	0	15	-.184	1	-.785	1
31		16	max	99.654	1	61.733	3	60.337	1	.014	1	-.005	12	.994	3
32			min	4.045	15	-44.049	1	2.449	15	0	15	-.144	1	-.812	1
33		17	max	99.654	1	224.395	3	106.989	1	.014	1	0	3	.839	3
34			min	4.045	15	-181.234	1	4.33	15	0	15	-.053	1	-.69	1
35		18	max	99.654	1	387.057	3	153.642	1	.014	1	.088	1	.507	3
36			min	4.045	15	-318.419	1	6.21	15	0	15	.004	15	-.419	1
37		19	max	99.654	1	549.718	3	200.295	1	.014	1	.279	1	0	1
38			min	4.045	15	-455.605	1	8.091	15	0	15	.011	15	0	3
39	M14	1	max	54.917	1	497.327	1	-8.386	15	.009	3	.327	1	0	1
40			min	2.236	15	-431.372	3	-207.603	1	-.013	1	.013	15	0	3
41		2	max	54.917	1	360.141	1	-6.505	15	.009	3	.127	1	.401	3
42			min	2.236	15	-309.224	3	-160.95	1	-.013	1	.005	15	-.464	1
43		3	max	54.917	1	222.956	1	-4.624	15	.009	3	.002	3	.67	3
44			min	2.236	15	-187.077	3	-114.297	1	-.013	1	-.022	1	-.78	1
45		4	max	54.917	1	85.771	1	-2.744	15	.009	3	-.004	12	.806	3
46			min	2.236	15	-64.929	3	-67.645	1	-.013	1	-.12	1	-.948	1
47		5	max	54.917	1	57.218	3	-.863	15	.009	3	-.007	12	.811	3
48			min	2.236	15	-51.415	1	-20.992	1	-.013	1	-.168	1	-.966	1
49		6	max	54.917	1	179.365	3	25.661	1	.009	3	-.007	15	.683	3
50			min	2.236	15	-188.6	1	.365	12	-.013	1	-.166	1	-.836	1
51		7	max	54.917	1	301.513	3	72.313	1	.009	3	-.005	15	.422	3
52			min	2.236	15	-325.785	1	2.276	12	-.013	1	-.113	1	-.558	1
53		8	max	54.917	1	423.66	3	118.966	1	.009	3	0	10	.029	3
54			min	2.236	15	-462.971	1	4.188	12	-.013	1	-.009	1	-.13	1
55		9	max	54.917	1	545.807	3	165.619	1	.009	3	.145	1	.446	1
56			min	2.236	15	-600.156	1	6.099	12	-.013	1	.003	12	-.496	3
57		10	max	54.917	1	737.341	1	-8.011	12	.013	1	.35	1	1.17	1
58			min	2.236	15	-667.955	3	-212.272	1	-.009	3	.011	12	-1.153	3
59		11	max	54.917	1	600.156	1	-6.099	12	.013	1	.145	1	.446	1
60			min	2.236	15	-545.807	3	-165.619	1	-.009	3	.003	12	-.496	3
61		12	max	54.917	1	462.971	1	-4.188	12	.013	1	0	10	.029	3
62			min	2.236	15	-423.66	3	-118.966	1	-.009	3	-.009	1	-.13	1
63		13	max	54.917	1	325.785	1	-2.276	12	.013	1	-.005	15	.422	3
64			min	2.236	15	-301.513	3	-72.313	1	-.009	3	-.113	1	-.558	1
65		14	max	54.917	1	188.6	1	-.365	12	.013	1	-.007	15	.683	3
66			min	2.236	15	-179.365	3	-25.661	1	-.009	3	-.166	1	-.836	1
67		15	max	54.917	1	51.415	1	20.992	1	.013	1	-.007	12	.811	3
68			min	2.236	15	-57.218	3	.863	15	-.009	3	-.168	1	-.966	1
69		16	max	54.917	1	64.929	3	67.645	1	.013	1	-.004	12	.806	3
70			min	2.236	15	-85.771	1	2.744	15	-.009	3	-.12	1	-.948	1
71		17	max	54.917	1	187.077	3	114.297	1	.013	1	.002	3	.67	3
72			min	2.236	15	-222.956	1	4.624	15	-.009	3	-.022	1	-.78	1
73		18	max	54.917	1	309.224	3	160.95	1	.013	1	.127	1	.401	3
74			min	2.236	15	-360.141	1	6.505	15	-.009	3	.005	15	-.464	1
75		19	max	54.917	1	431.372	3	207.603	1	.013	1	.327	1	0	1
76			min	2.236	15	-497.327	1	8.386	15	-.009	3	.013	15	0	3
77	M15	1	max	-2.396	15	565.649	1	-8.382	15	.014	1	.326	1	0	2
78			min	-58.832	1	-226.669	3	-207.533	1	-.007	3	.013	15	0	3
79		2	max	-2.396	15	408.206	1	-6.501	15	.014	1	.127	1	.212	3
80			min	-58.832	1	-165.294	3	-160.88	1	-.007	3	.005	15	-.527	1
81		3	max	-2.396	15	250.763	1	-4.621	15	.014	1	.001	3	.358	3
82			min	-58.832	1	-103.918	3	-114.228	1	-.007	3	-.022	1	-.884	1
83		4	max	-2.396	15	93.321	1	-2.74	15	.014	1	-.004	12	.437	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-58.832	1	-42.543	3	-67.575	1	-.007	3	-.121	1	-1.071	1
85		5	max	-2.396	15	18.833	3	-.859	15	.014	1	-.007	12	.45	3
86			min	-58.832	1	-65.954	2	-20.922	1	-.007	3	-.169	1	-1.087	1
87		6	max	-2.396	15	80.209	3	25.731	1	.014	1	-.007	15	.397	3
88			min	-58.832	1	-221.564	1	.425	12	-.007	3	-.166	1	-.932	1
89		7	max	-2.396	15	141.584	3	72.383	1	.014	1	-.005	15	.277	3
90			min	-58.832	1	-379.007	1	2.336	12	-.007	3	-.113	1	-.607	1
91		8	max	-2.396	15	202.96	3	119.036	1	.014	1	0	10	.09	3
92			min	-58.832	1	-536.449	1	4.248	12	-.007	3	-.009	1	-.111	1
93		9	max	-2.396	15	264.335	3	165.689	1	.014	1	.145	1	.572	2
94			min	-58.832	1	-693.892	1	6.159	12	-.007	3	.004	12	-.163	3
95		10	max	-2.396	15	851.335	1	-8.071	12	.007	3	.35	1	1.4	2
96			min	-58.832	1	-325.711	3	-212.341	1	-.014	1	.011	12	-.483	3
97		11	max	-2.396	15	693.892	1	-6.159	12	.007	3	.145	1	.572	2
98			min	-58.832	1	-264.335	3	-165.689	1	-.014	1	.004	12	-.163	3
99		12	max	-2.396	15	536.449	1	-4.248	12	.007	3	0	10	.09	3
100			min	-58.832	1	-202.96	3	-119.036	1	-.014	1	-.009	1	-.111	1
101		13	max	-2.396	15	379.007	1	-2.336	12	.007	3	-.005	15	.277	3
102			min	-58.832	1	-141.584	3	-72.383	1	-.014	1	-.113	1	-.607	1
103		14	max	-2.396	15	221.564	1	-.425	12	.007	3	-.007	15	.397	3
104			min	-58.832	1	-80.209	3	-25.731	1	-.014	1	-.166	1	-.932	1
105		15	max	-2.396	15	65.954	2	20.922	1	.007	3	-.007	12	.45	3
106			min	-58.832	1	-18.833	3	.859	15	-.014	1	-.169	1	-1.087	1
107		16	max	-2.396	15	42.543	3	67.575	1	.007	3	-.004	12	.437	3
108			min	-58.832	1	-93.321	1	2.74	15	-.014	1	-.121	1	-1.071	1
109		17	max	-2.396	15	103.918	3	114.228	1	.007	3	.001	3	.358	3
110			min	-58.832	1	-250.763	1	4.621	15	-.014	1	-.022	1	-.884	1
111		18	max	-2.396	15	165.294	3	160.88	1	.007	3	.127	1	.212	3
112			min	-58.832	1	-408.206	1	6.501	15	-.014	1	.005	15	-.527	1
113		19	max	-2.396	15	226.669	3	207.533	1	.007	3	.326	1	0	2
114			min	-58.832	1	-565.649	1	8.382	15	-.014	1	.013	15	0	3
115	M16	1	max	-4.529	15	524.372	1	-8.106	15	.012	1	.282	1	0	2
116			min	-111.299	1	-201.825	3	-200.761	1	-.01	3	.011	15	0	3
117		2	max	-4.529	15	366.93	1	-6.226	15	.012	1	.09	1	.185	3
118			min	-111.299	1	-140.449	3	-154.108	1	-.01	3	.004	15	-.483	1
119		3	max	-4.529	15	209.487	1	-4.345	15	.012	1	0	12	.304	3
120			min	-111.299	1	-79.073	3	-107.455	1	-.01	3	-.052	1	-.795	1
121		4	max	-4.529	15	52.517	2	-2.464	15	.012	1	-.005	12	.357	3
122			min	-111.299	1	-17.698	3	-60.802	1	-.01	3	-.143	1	-.937	1
123		5	max	-4.529	15	43.678	3	-.584	15	.012	1	-.007	12	.343	3
124			min	-111.299	1	-105.398	1	-14.15	1	-.01	3	-.183	1	-.908	1
125		6	max	-4.529	15	105.053	3	32.503	1	.012	1	-.007	15	.262	3
126			min	-111.299	1	-262.841	1	.84	12	-.01	3	-.173	1	-.708	1
127		7	max	-4.529	15	166.429	3	79.156	1	.012	1	-.005	15	.115	3
128			min	-111.299	1	-420.283	1	2.752	12	-.01	3	-.113	1	-.341	2
129		8	max	-4.529	15	227.805	3	125.808	1	.012	1	0	10	.202	1
130			min	-111.299	1	-577.726	1	4.663	12	-.01	3	-.003	3	-.099	3
131		9	max	-4.529	15	289.18	3	172.461	1	.012	1	.16	1	.913	1
132			min	-111.299	1	-735.168	1	6.575	12	-.01	3	.005	12	-.379	3
133		10	max	-4.529	15	892.611	1	-8.486	12	.012	1	.372	1	1.795	1
134			min	-111.299	1	-350.556	3	-219.114	1	-.01	3	.013	12	-.725	3
135		11	max	-4.529	15	735.168	1	-6.575	12	.01	3	.16	1	.913	1
136			min	-111.299	1	-289.18	3	-172.461	1	-.012	1	.005	12	-.379	3
137		12	max	-4.529	15	577.726	1	-4.663	12	.01	3	0	10	.202	1
138			min	-111.299	1	-227.805	3	-125.808	1	-.012	1	-.003	3	-.099	3
139		13	max	-4.529	15	420.283	1	-2.752	12	.01	3	-.005	15	.115	3
140			min	-111.299	1	-166.429	3	-79.156	1	-.012	1	-.113	1	-.341	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-4.529	15	262.841	1	-.84	12	.01	3	-.007	15	.262	3
142		min	-111.299	1	-105.053	3	-32.503	1	-.012	1	-.173	1	-.708	1
143	15	max	-4.529	15	105.398	1	14.15	1	.01	3	-.007	12	.343	3
144		min	-111.299	1	-43.678	3	.584	15	-.012	1	-.183	1	-.908	1
145	16	max	-4.529	15	17.698	3	60.802	1	.01	3	-.005	12	.357	3
146		min	-111.299	1	-52.517	2	2.464	15	-.012	1	-.143	1	-.937	1
147	17	max	-4.529	15	79.073	3	107.455	1	.01	3	0	12	.304	3
148		min	-111.299	1	-209.487	1	4.345	15	-.012	1	-.052	1	-.795	1
149	18	max	-4.529	15	140.449	3	154.108	1	.01	3	.09	1	.185	3
150		min	-111.299	1	-366.93	1	6.226	15	-.012	1	.004	15	-.483	1
151	19	max	-4.529	15	201.825	3	200.761	1	.01	3	.282	1	0	2
152		min	-111.299	1	-524.372	1	8.106	15	-.012	1	.011	15	0	3
153	M2	1	max	1057.912	1	2.022	4	.797	1	0	5	0	3	1
154		min	-1012.963	3	.476	15	.032	15	0	1	0	1	0	1
155	2	max	1058.386	1	1.985	4	.797	1	0	5	0	1	0	15
156		min	-1012.608	3	.467	15	.032	15	0	1	0	15	0	4
157	3	max	1058.859	1	1.948	4	.797	1	0	5	0	1	0	15
158		min	-1012.252	3	.458	15	.032	15	0	1	0	15	-.001	4
159	4	max	1059.333	1	1.911	4	.797	1	0	5	0	1	0	15
160		min	-1011.897	3	.45	15	.032	15	0	1	0	15	-.002	4
161	5	max	1059.807	1	1.874	4	.797	1	0	5	.001	1	0	15
162		min	-1011.542	3	.441	15	.032	15	0	1	0	15	-.002	4
163	6	max	1060.281	1	1.837	4	.797	1	0	5	.001	1	0	15
164		min	-1011.186	3	.432	15	.032	15	0	1	0	15	-.003	4
165	7	max	1060.754	1	1.8	4	.797	1	0	5	.002	1	0	15
166		min	-1010.831	3	.423	15	.032	15	0	1	0	15	-.004	4
167	8	max	1061.228	1	1.763	4	.797	1	0	5	.002	1	0	15
168		min	-1010.476	3	.415	15	.032	15	0	1	0	15	-.004	4
169	9	max	1061.702	1	1.726	4	.797	1	0	5	.002	1	-.001	15
170		min	-1010.121	3	.406	15	.032	15	0	1	0	15	-.005	4
171	10	max	1062.176	1	1.689	4	.797	1	0	5	.002	1	-.001	15
172		min	-1009.765	3	.397	15	.032	15	0	1	0	15	-.005	4
173	11	max	1062.649	1	1.652	4	.797	1	0	5	.003	1	-.001	15
174		min	-1009.41	3	.389	15	.032	15	0	1	0	15	-.006	4
175	12	max	1063.123	1	1.615	4	.797	1	0	5	.003	1	-.002	15
176		min	-1009.055	3	.38	15	.032	15	0	1	0	15	-.006	4
177	13	max	1063.597	1	1.578	4	.797	1	0	5	.003	1	-.002	15
178		min	-1008.699	3	.371	15	.032	15	0	1	0	15	-.007	4
179	14	max	1064.071	1	1.541	4	.797	1	0	5	.003	1	-.002	15
180		min	-1008.344	3	.363	15	.032	15	0	1	0	15	-.007	4
181	15	max	1064.544	1	1.504	4	.797	1	0	5	.004	1	-.002	15
182		min	-1007.989	3	.354	15	.032	15	0	1	0	15	-.008	4
183	16	max	1065.018	1	1.467	4	.797	1	0	5	.004	1	-.002	15
184		min	-1007.633	3	.345	15	.032	15	0	1	0	15	-.008	4
185	17	max	1065.492	1	1.43	4	.797	1	0	5	.004	1	-.002	15
186		min	-1007.278	3	.336	15	.032	15	0	1	0	15	-.009	4
187	18	max	1065.966	1	1.393	4	.797	1	0	5	.004	1	-.002	15
188		min	-1006.923	3	.328	15	.032	15	0	1	0	15	-.009	4
189	19	max	1066.439	1	1.356	4	.797	1	0	5	.005	1	-.002	15
190		min	-1006.568	3	.319	15	.032	15	0	1	0	15	-.01	4
191	M3	1	max	452.717	2	8.992	4	.343	1	0	12	0	.01	4
192		min	-600.879	3	2.114	15	.014	15	0	1	0	15	.002	15
193	2	max	452.547	2	8.12	4	.343	1	0	12	0	1	.006	4
194		min	-601.007	3	1.909	15	.014	15	0	1	0	15	.001	15
195	3	max	452.377	2	7.248	4	.343	1	0	12	0	1	.003	2
196		min	-601.135	3	1.704	15	.014	15	0	1	0	15	0	3
197	4	max	452.206	2	6.376	4	.343	1	0	12	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-601.263	3	1.499	15	.014	15	0	1	0	15	-.002	3
199		5	max	452.036	2	5.504	4	.343	1	0	12	0	1	0	15
200			min	-601.39	3	1.294	15	.014	15	0	1	0	15	-.004	4
201		6	max	451.866	2	4.632	4	.343	1	0	12	.001	1	-.001	15
202			min	-601.518	3	1.089	15	.014	15	0	1	0	15	-.006	4
203		7	max	451.695	2	3.76	4	.343	1	0	12	.001	1	-.002	15
204			min	-601.646	3	.884	15	.014	15	0	1	0	15	-.008	4
205		8	max	451.525	2	2.888	4	.343	1	0	12	.001	1	-.002	15
206			min	-601.774	3	.679	15	.014	15	0	1	0	15	-.01	4
207		9	max	451.355	2	2.016	4	.343	1	0	12	.002	1	-.003	15
208			min	-601.902	3	.474	15	.014	15	0	1	0	15	-.011	4
209		10	max	451.184	2	1.144	4	.343	1	0	12	.002	1	-.003	15
210			min	-602.029	3	.269	15	.014	15	0	1	0	15	-.012	4
211		11	max	451.014	2	.332	2	.343	1	0	12	.002	1	-.003	15
212			min	-602.157	3	-.007	3	.014	15	0	1	0	15	-.012	4
213		12	max	450.844	2	-.141	15	.343	1	0	12	.002	1	-.003	15
214			min	-602.285	3	-.6	4	.014	15	0	1	0	15	-.012	4
215		13	max	450.673	2	-.346	15	.343	1	0	12	.002	1	-.003	15
216			min	-602.413	3	-1.472	4	.014	15	0	1	0	15	-.012	4
217		14	max	450.503	2	-.551	15	.343	1	0	12	.002	1	-.003	15
218			min	-602.54	3	-2.344	4	.014	15	0	1	0	15	-.011	4
219		15	max	450.333	2	-.756	15	.343	1	0	12	.003	1	-.002	15
220			min	-602.668	3	-3.216	4	.014	15	0	1	0	15	-.009	4
221		16	max	450.162	2	-.961	15	.343	1	0	12	.003	1	-.002	15
222			min	-602.796	3	-4.088	4	.014	15	0	1	0	15	-.008	4
223		17	max	449.992	2	-1.166	15	.343	1	0	12	.003	1	-.001	15
224			min	-602.924	3	-4.96	4	.014	15	0	1	0	15	-.006	4
225		18	max	449.822	2	-1.371	15	.343	1	0	12	.003	1	0	15
226			min	-603.051	3	-5.832	4	.014	15	0	1	0	15	-.003	4
227		19	max	449.651	2	-1.576	15	.343	1	0	12	.003	1	0	1
228			min	-603.179	3	-6.704	4	.014	15	0	1	0	15	0	1
229	M4	1	max	1161.836	1	0	1	-.691	15	0	1	.002	1	0	1
230			min	-70.051	3	0	1	-17.075	1	0	1	0	15	0	1
231		2	max	1162.006	1	0	1	-.691	15	0	1	0	1	0	1
232			min	-69.924	3	0	1	-17.075	1	0	1	0	15	0	1
233		3	max	1162.176	1	0	1	-.691	15	0	1	0	15	0	1
234			min	-69.796	3	0	1	-17.075	1	0	1	-.002	1	0	1
235		4	max	1162.347	1	0	1	-.691	15	0	1	0	15	0	1
236			min	-69.668	3	0	1	-17.075	1	0	1	-.004	1	0	1
237		5	max	1162.517	1	0	1	-.691	15	0	1	0	15	0	1
238			min	-69.54	3	0	1	-17.075	1	0	1	-.006	1	0	1
239		6	max	1162.687	1	0	1	-.691	15	0	1	0	15	0	1
240			min	-69.413	3	0	1	-17.075	1	0	1	-.008	1	0	1
241		7	max	1162.858	1	0	1	-.691	15	0	1	0	15	0	1
242			min	-69.285	3	0	1	-17.075	1	0	1	-.01	1	0	1
243		8	max	1163.028	1	0	1	-.691	15	0	1	0	15	0	1
244			min	-69.157	3	0	1	-17.075	1	0	1	-.012	1	0	1
245		9	max	1163.198	1	0	1	-.691	15	0	1	0	15	0	1
246			min	-69.029	3	0	1	-17.075	1	0	1	-.013	1	0	1
247		10	max	1163.369	1	0	1	-.691	15	0	1	0	15	0	1
248			min	-68.902	3	0	1	-17.075	1	0	1	-.015	1	0	1
249		11	max	1163.539	1	0	1	-.691	15	0	1	0	15	0	1
250			min	-68.774	3	0	1	-17.075	1	0	1	-.017	1	0	1
251		12	max	1163.709	1	0	1	-.691	15	0	1	0	15	0	1
252			min	-68.646	3	0	1	-17.075	1	0	1	-.019	1	0	1
253		13	max	1163.88	1	0	1	-.691	15	0	1	0	15	0	1
254			min	-68.518	3	0	1	-17.075	1	0	1	-.021	1	0	1





Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1164.05	1	0	1	-.691	15	0	1	0	15	0	1
256		min	-68.39	3	0	1	-17.075	1	0	1	-.023	1	0	1
257	15	max	1164.221	1	0	1	-.691	15	0	1	-.001	15	0	1
258		min	-68.263	3	0	1	-17.075	1	0	1	-.025	1	0	1
259	16	max	1164.391	1	0	1	-.691	15	0	1	-.001	15	0	1
260		min	-68.135	3	0	1	-17.075	1	0	1	-.027	1	0	1
261	17	max	1164.561	1	0	1	-.691	15	0	1	-.001	15	0	1
262		min	-68.007	3	0	1	-17.075	1	0	1	-.029	1	0	1
263	18	max	1164.732	1	0	1	-.691	15	0	1	-.001	15	0	1
264		min	-67.879	3	0	1	-17.075	1	0	1	-.031	1	0	1
265	19	max	1164.902	1	0	1	-.691	15	0	1	-.001	15	0	1
266		min	-67.752	3	0	1	-17.075	1	0	1	-.033	1	0	1
267	M6	1	max	3343.058	1	2.226	2	0	1	0	0	1	0	1
268		min	-3275.134	3	.326	12	0	1	0	1	0	1	0	1
269	2	max	3343.532	1	2.197	2	0	1	0	1	0	1	0	12
270		min	-3274.779	3	.311	12	0	1	0	1	0	1	0	2
271	3	max	3344.005	1	2.168	2	0	1	0	1	0	1	0	12
272		min	-3274.423	3	.297	12	0	1	0	1	0	1	-.001	2
273	4	max	3344.479	1	2.139	2	0	1	0	1	0	1	0	12
274		min	-3274.068	3	.282	12	0	1	0	1	0	1	-.002	2
275	5	max	3344.953	1	2.11	2	0	1	0	1	0	1	0	12
276		min	-3273.713	3	.268	12	0	1	0	1	0	1	-.003	2
277	6	max	3345.427	1	2.081	2	0	1	0	1	0	1	0	12
278		min	-3273.358	3	.254	12	0	1	0	1	0	1	-.003	2
279	7	max	3345.9	1	2.052	2	0	1	0	1	0	1	0	12
280		min	-3273.002	3	.239	12	0	1	0	1	0	1	-.004	2
281	8	max	3346.374	1	2.023	2	0	1	0	1	0	1	0	12
282		min	-3272.647	3	.225	12	0	1	0	1	0	1	-.005	2
283	9	max	3346.848	1	1.995	2	0	1	0	1	0	1	0	12
284		min	-3272.292	3	.21	12	0	1	0	1	0	1	-.005	2
285	10	max	3347.321	1	1.966	2	0	1	0	1	0	1	0	12
286		min	-3271.936	3	.196	12	0	1	0	1	0	1	-.006	2
287	11	max	3347.795	1	1.937	2	0	1	0	1	0	1	0	12
288		min	-3271.581	3	.181	12	0	1	0	1	0	1	-.007	2
289	12	max	3348.269	1	1.908	2	0	1	0	1	0	1	0	12
290		min	-3271.226	3	.167	12	0	1	0	1	0	1	-.007	2
291	13	max	3348.743	1	1.879	2	0	1	0	1	0	1	0	12
292		min	-3270.87	3	.151	3	0	1	0	1	0	1	-.008	2
293	14	max	3349.216	1	1.85	2	0	1	0	1	0	1	0	12
294		min	-3270.515	3	.129	3	0	1	0	1	0	1	-.008	2
295	15	max	3349.69	1	1.821	2	0	1	0	1	0	1	-.001	12
296		min	-3270.16	3	.107	3	0	1	0	1	0	1	-.009	2
297	16	max	3350.164	1	1.793	2	0	1	0	1	0	1	-.001	12
298		min	-3269.805	3	.086	3	0	1	0	1	0	1	-.01	2
299	17	max	3350.638	1	1.764	2	0	1	0	1	0	1	-.001	12
300		min	-3269.449	3	.064	3	0	1	0	1	0	1	-.01	2
301	18	max	3351.111	1	1.735	2	0	1	0	1	0	1	-.001	12
302		min	-3269.094	3	.042	3	0	1	0	1	0	1	-.011	2
303	19	max	3351.585	1	1.706	2	0	1	0	1	0	1	-.001	12
304		min	-3268.739	3	.021	3	0	1	0	1	0	1	-.011	2
305	M7	1	max	1769.504	2	9.031	4	0	1	0	0	1	.011	2
306		min	-1851.407	3	2.12	15	0	1	0	1	0	1	.001	12
307	2	max	1769.334	2	8.159	4	0	1	0	1	0	1	.008	2
308		min	-1851.534	3	1.915	15	0	1	0	1	0	1	0	3
309	3	max	1769.163	2	7.287	4	0	1	0	1	0	1	.005	2
310		min	-1851.662	3	1.71	15	0	1	0	1	0	1	-.003	3
311	4	max	1768.993	2	6.415	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
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Nov 4, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-1851.79	3	1.505	15	0	1	0	1	0	1	-.004	3
313	5	max	1768.823	2	5.543	4	0	1	0	1	0	1	0	2
314		min	-1851.918	3	1.3	15	0	1	0	1	0	1	-.006	3
315	6	max	1768.652	2	4.671	4	0	1	0	1	0	1	-.001	15
316		min	-1852.045	3	1.095	15	0	1	0	1	0	1	-.007	3
317	7	max	1768.482	2	3.799	4	0	1	0	1	0	1	-.002	15
318		min	-1852.173	3	.89	15	0	1	0	1	0	1	-.008	4
319	8	max	1768.312	2	2.927	4	0	1	0	1	0	1	-.002	15
320		min	-1852.301	3	.685	15	0	1	0	1	0	1	-.01	4
321	9	max	1768.141	2	2.055	4	0	1	0	1	0	1	-.003	15
322		min	-1852.429	3	.472	12	0	1	0	1	0	1	-.011	4
323	10	max	1767.971	2	1.332	2	0	1	0	1	0	1	-.003	15
324		min	-1852.556	3	.133	12	0	1	0	1	0	1	-.012	4
325	11	max	1767.801	2	.652	2	0	1	0	1	0	1	-.003	15
326		min	-1852.684	3	-.364	3	0	1	0	1	0	1	-.012	4
327	12	max	1767.63	2	-.027	2	0	1	0	1	0	1	-.003	15
328		min	-1852.812	3	-.873	3	0	1	0	1	0	1	-.012	4
329	13	max	1767.46	2	-.34	15	0	1	0	1	0	1	-.003	15
330		min	-1852.94	3	-1.433	4	0	1	0	1	0	1	-.011	4
331	14	max	1767.29	2	-.545	15	0	1	0	1	0	1	-.002	15
332		min	-1853.068	3	-2.305	4	0	1	0	1	0	1	-.011	4
333	15	max	1767.119	2	-.75	15	0	1	0	1	0	1	-.002	15
334		min	-1853.195	3	-3.177	4	0	1	0	1	0	1	-.009	4
335	16	max	1766.949	2	-.955	15	0	1	0	1	0	1	-.002	15
336		min	-1853.323	3	-4.049	4	0	1	0	1	0	1	-.008	4
337	17	max	1766.779	2	-1.16	15	0	1	0	1	0	1	-.001	15
338		min	-1853.451	3	-4.921	4	0	1	0	1	0	1	-.005	4
339	18	max	1766.608	2	-1.365	15	0	1	0	1	0	1	0	15
340		min	-1853.579	3	-5.793	4	0	1	0	1	0	1	-.003	4
341	19	max	1766.438	2	-1.57	15	0	1	0	1	0	1	0	1
342		min	-1853.706	3	-6.665	4	0	1	0	1	0	1	0	1
343	M8	1	max	3090.536	1	0	1	0	1	0	1	0	1	1
344		min	-305.637	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3090.707	1	0	1	0	1	0	1	0	1	0	1
346		min	-305.509	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3090.877	1	0	1	0	1	0	1	0	1	0	1
348		min	-305.381	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3091.047	1	0	1	0	1	0	1	0	1	0	1
350		min	-305.254	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3091.218	1	0	1	0	1	0	1	0	1	0	1
352		min	-305.126	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3091.388	1	0	1	0	1	0	1	0	1	0	1
354		min	-304.998	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3091.559	1	0	1	0	1	0	1	0	1	0	1
356		min	-304.87	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3091.729	1	0	1	0	1	0	1	0	1	0	1
358		min	-304.743	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3091.899	1	0	1	0	1	0	1	0	1	0	1
360		min	-304.615	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3092.07	1	0	1	0	1	0	1	0	1	0	1
362		min	-304.487	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3092.24	1	0	1	0	1	0	1	0	1	0	1
364		min	-304.359	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3092.41	1	0	1	0	1	0	1	0	1	0	1
366		min	-304.232	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3092.581	1	0	1	0	1	0	1	0	1	0	1
368		min	-304.104	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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Job Number :  
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Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3092.751	1	0	1	0	1	0	1	0	1	0	1
370			min	-303.976	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3092.921	1	0	1	0	1	0	1	0	1	0	1
372			min	-303.848	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3093.092	1	0	1	0	1	0	1	0	1	0	1
374			min	-303.72	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3093.262	1	0	1	0	1	0	1	0	1	0	1
376			min	-303.593	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3093.432	1	0	1	0	1	0	1	0	1	0	1
378			min	-303.465	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3093.603	1	0	1	0	1	0	1	0	1	0	1
380			min	-303.337	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1057.912	1	2.022	4	-.032	15	0	1	0	1	0	1
382			min	-1012.963	3	.476	15	-.797	1	0	5	0	3	0	1
383		2	max	1058.386	1	1.985	4	-.032	15	0	1	0	15	0	15
384			min	-1012.608	3	.467	15	-.797	1	0	5	0	1	0	4
385		3	max	1058.859	1	1.948	4	-.032	15	0	1	0	15	0	15
386			min	-1012.252	3	.458	15	-.797	1	0	5	0	1	-.001	4
387		4	max	1059.333	1	1.911	4	-.032	15	0	1	0	15	0	15
388			min	-1011.897	3	.45	15	-.797	1	0	5	0	1	-.002	4
389		5	max	1059.807	1	1.874	4	-.032	15	0	1	0	15	0	15
390			min	-1011.542	3	.441	15	-.797	1	0	5	-.001	1	-.002	4
391		6	max	1060.281	1	1.837	4	-.032	15	0	1	0	15	0	15
392			min	-1011.186	3	.432	15	-.797	1	0	5	-.001	1	-.003	4
393		7	max	1060.754	1	1.8	4	-.032	15	0	1	0	15	0	15
394			min	-1010.831	3	.423	15	-.797	1	0	5	-.002	1	-.004	4
395		8	max	1061.228	1	1.763	4	-.032	15	0	1	0	15	0	15
396			min	-1010.476	3	.415	15	-.797	1	0	5	-.002	1	-.004	4
397		9	max	1061.702	1	1.726	4	-.032	15	0	1	0	15	-.001	15
398			min	-1010.121	3	.406	15	-.797	1	0	5	-.002	1	-.005	4
399		10	max	1062.176	1	1.689	4	-.032	15	0	1	0	15	-.001	15
400			min	-1009.765	3	.397	15	-.797	1	0	5	-.002	1	-.005	4
401		11	max	1062.649	1	1.652	4	-.032	15	0	1	0	15	-.001	15
402			min	-1009.41	3	.389	15	-.797	1	0	5	-.003	1	-.006	4
403		12	max	1063.123	1	1.615	4	-.032	15	0	1	0	15	-.002	15
404			min	-1009.055	3	.38	15	-.797	1	0	5	-.003	1	-.006	4
405		13	max	1063.597	1	1.578	4	-.032	15	0	1	0	15	-.002	15
406			min	-1008.699	3	.371	15	-.797	1	0	5	-.003	1	-.007	4
407		14	max	1064.071	1	1.541	4	-.032	15	0	1	0	15	-.002	15
408			min	-1008.344	3	.363	15	-.797	1	0	5	-.003	1	-.007	4
409		15	max	1064.544	1	1.504	4	-.032	15	0	1	0	15	-.002	15
410			min	-1007.989	3	.354	15	-.797	1	0	5	-.004	1	-.008	4
411		16	max	1065.018	1	1.467	4	-.032	15	0	1	0	15	-.002	15
412			min	-1007.633	3	.345	15	-.797	1	0	5	-.004	1	-.008	4
413		17	max	1065.492	1	1.43	4	-.032	15	0	1	0	15	-.002	15
414			min	-1007.278	3	.336	15	-.797	1	0	5	-.004	1	-.009	4
415		18	max	1065.966	1	1.393	4	-.032	15	0	1	0	15	-.002	15
416			min	-1006.923	3	.328	15	-.797	1	0	5	-.004	1	-.009	4
417		19	max	1066.439	1	1.356	4	-.032	15	0	1	0	15	-.002	15
418			min	-1006.568	3	.319	15	-.797	1	0	5	-.005	1	-.01	4
419	M11	1	max	452.717	2	8.992	4	-.014	15	0	1	0	15	.01	4
420			min	-600.879	3	2.114	15	-.343	1	0	12	0	1	.002	15
421		2	max	452.547	2	8.12	4	-.014	15	0	1	0	15	.006	4
422			min	-601.007	3	1.909	15	-.343	1	0	12	0	1	.001	15
423		3	max	452.377	2	7.248	4	-.014	15	0	1	0	15	.003	2
424			min	-601.135	3	1.704	15	-.343	1	0	12	0	1	0	3
425		4	max	452.206	2	6.376	4	-.014	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-601.263	3	1.499	15	-.343	1	0	12	0	1	-.002	3
427		5	max	452.036	2	5.504	4	-.014	15	0	1	0	15	0	15
428			min	-601.39	3	1.294	15	-.343	1	0	12	0	1	-.004	4
429		6	max	451.866	2	4.632	4	-.014	15	0	1	0	15	-.001	15
430			min	-601.518	3	1.089	15	-.343	1	0	12	-.001	1	-.006	4
431		7	max	451.695	2	3.76	4	-.014	15	0	1	0	15	-.002	15
432			min	-601.646	3	.884	15	-.343	1	0	12	-.001	1	-.008	4
433		8	max	451.525	2	2.888	4	-.014	15	0	1	0	15	-.002	15
434			min	-601.774	3	.679	15	-.343	1	0	12	-.001	1	-.01	4
435		9	max	451.355	2	2.016	4	-.014	15	0	1	0	15	-.003	15
436			min	-601.902	3	.474	15	-.343	1	0	12	-.002	1	-.011	4
437		10	max	451.184	2	1.144	4	-.014	15	0	1	0	15	-.003	15
438			min	-602.029	3	.269	15	-.343	1	0	12	-.002	1	-.012	4
439		11	max	451.014	2	.332	2	-.014	15	0	1	0	15	-.003	15
440			min	-602.157	3	-.007	3	-.343	1	0	12	-.002	1	-.012	4
441		12	max	450.844	2	-.141	15	-.014	15	0	1	0	15	-.003	15
442			min	-602.285	3	-.6	4	-.343	1	0	12	-.002	1	-.012	4
443		13	max	450.673	2	-.346	15	-.014	15	0	1	0	15	-.003	15
444			min	-602.413	3	-1.472	4	-.343	1	0	12	-.002	1	-.012	4
445		14	max	450.503	2	-.551	15	-.014	15	0	1	0	15	-.003	15
446			min	-602.54	3	-2.344	4	-.343	1	0	12	-.002	1	-.011	4
447		15	max	450.333	2	-.756	15	-.014	15	0	1	0	15	-.002	15
448			min	-602.668	3	-3.216	4	-.343	1	0	12	-.003	1	-.009	4
449		16	max	450.162	2	-.961	15	-.014	15	0	1	0	15	-.002	15
450			min	-602.796	3	-4.088	4	-.343	1	0	12	-.003	1	-.008	4
451		17	max	449.992	2	-1.166	15	-.014	15	0	1	0	15	-.001	15
452			min	-602.924	3	-4.96	4	-.343	1	0	12	-.003	1	-.006	4
453		18	max	449.822	2	-1.371	15	-.014	15	0	1	0	15	0	15
454			min	-603.051	3	-5.832	4	-.343	1	0	12	-.003	1	-.003	4
455		19	max	449.651	2	-1.576	15	-.014	15	0	1	0	15	0	1
456			min	-603.179	3	-6.704	4	-.343	1	0	12	-.003	1	0	1
457	M12	1	max	1161.836	1	0	1	17.075	1	0	1	0	15	0	1
458			min	-70.051	3	0	1	.691	15	0	1	-.002	1	0	1
459		2	max	1162.006	1	0	1	17.075	1	0	1	0	15	0	1
460			min	-69.924	3	0	1	.691	15	0	1	0	1	0	1
461		3	max	1162.176	1	0	1	17.075	1	0	1	.002	1	0	1
462			min	-69.796	3	0	1	.691	15	0	1	0	15	0	1
463		4	max	1162.347	1	0	1	17.075	1	0	1	.004	1	0	1
464			min	-69.668	3	0	1	.691	15	0	1	0	15	0	1
465		5	max	1162.517	1	0	1	17.075	1	0	1	.006	1	0	1
466			min	-69.54	3	0	1	.691	15	0	1	0	15	0	1
467		6	max	1162.687	1	0	1	17.075	1	0	1	.008	1	0	1
468			min	-69.413	3	0	1	.691	15	0	1	0	15	0	1
469		7	max	1162.858	1	0	1	17.075	1	0	1	.01	1	0	1
470			min	-69.285	3	0	1	.691	15	0	1	0	15	0	1
471		8	max	1163.028	1	0	1	17.075	1	0	1	.012	1	0	1
472			min	-69.157	3	0	1	.691	15	0	1	0	15	0	1
473		9	max	1163.198	1	0	1	17.075	1	0	1	.013	1	0	1
474			min	-69.029	3	0	1	.691	15	0	1	0	15	0	1
475		10	max	1163.369	1	0	1	17.075	1	0	1	.015	1	0	1
476			min	-68.902	3	0	1	.691	15	0	1	0	15	0	1
477		11	max	1163.539	1	0	1	17.075	1	0	1	.017	1	0	1
478			min	-68.774	3	0	1	.691	15	0	1	0	15	0	1
479		12	max	1163.709	1	0	1	17.075	1	0	1	.019	1	0	1
480			min	-68.646	3	0	1	.691	15	0	1	0	15	0	1
481		13	max	1163.88	1	0	1	17.075	1	0	1	.021	1	0	1
482			min	-68.518	3	0	1	.691	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1164.05	1	0	1	17.075	1	0	1	.023	1	0	1
484			min	-68.39	3	0	1	.691	15	0	1	0	15	0	1
485		15	max	1164.221	1	0	1	17.075	1	0	1	.025	1	0	1
486			min	-68.263	3	0	1	.691	15	0	1	.001	15	0	1
487		16	max	1164.391	1	0	1	17.075	1	0	1	.027	1	0	1
488			min	-68.135	3	0	1	.691	15	0	1	.001	15	0	1
489		17	max	1164.561	1	0	1	17.075	1	0	1	.029	1	0	1
490			min	-68.007	3	0	1	.691	15	0	1	.001	15	0	1
491		18	max	1164.732	1	0	1	17.075	1	0	1	.031	1	0	1
492			min	-67.879	3	0	1	.691	15	0	1	.001	15	0	1
493		19	max	1164.902	1	0	1	17.075	1	0	1	.033	1	0	1
494			min	-67.752	3	0	1	.691	15	0	1	.001	15	0	1
495	M1	1	max	200.301	1	549.684	3	-4.045	15	0	1	.279	1	0	15
496			min	8.091	15	-453.249	1	-99.464	1	0	3	.011	15	-.014	1
497		2	max	201.013	1	548.539	3	-4.045	15	0	1	.218	1	.268	1
498			min	8.306	15	-454.776	1	-99.464	1	0	3	.009	15	-.342	3
499		3	max	386.713	3	523.514	1	-4.013	15	0	3	.156	1	.54	1
500			min	-249.82	2	-397.194	3	-98.996	1	0	1	.006	15	-.672	3
501		4	max	387.247	3	521.987	1	-4.013	15	0	3	.095	1	.215	1
502			min	-249.108	2	-398.339	3	-98.996	1	0	1	.004	15	-.425	3
503		5	max	387.781	3	520.46	1	-4.013	15	0	3	.033	1	-.005	15
504			min	-248.396	2	-399.484	3	-98.996	1	0	1	.001	15	-.177	3
505		6	max	388.315	3	518.933	1	-4.013	15	0	3	-.001	15	.071	3
506			min	-247.684	2	-400.629	3	-98.996	1	0	1	-.028	1	-.431	1
507		7	max	388.849	3	517.406	1	-4.013	15	0	3	-.004	15	.32	3
508			min	-246.972	2	-401.775	3	-98.996	1	0	1	-.09	1	-.753	1
509		8	max	389.383	3	515.879	1	-4.013	15	0	3	-.006	15	.57	3
510			min	-246.26	2	-402.92	3	-98.996	1	0	1	-.151	1	-1.073	1
511		9	max	403.815	3	35.151	2	-6.249	15	0	9	.094	1	.668	3
512			min	-162.27	2	.466	15	-153.973	1	0	3	.004	15	-1.222	1
513		10	max	404.349	3	33.624	2	-6.249	15	0	9	0	15	.649	3
514			min	-161.557	2	.005	15	-153.973	1	0	3	-.002	1	-1.234	1
515		11	max	404.883	3	32.097	2	-6.249	15	0	9	-.004	15	.632	3
516			min	-160.845	2	-1.828	4	-153.973	1	0	3	-.097	1	-1.245	1
517		12	max	419.217	3	255.933	3	-3.854	15	0	1	.148	1	.551	3
518			min	-90.11	10	-554.008	1	-95.217	1	0	3	.006	15	-1.1	1
519		13	max	419.751	3	254.787	3	-3.854	15	0	1	.089	1	.393	3
520			min	-89.516	10	-555.535	1	-95.217	1	0	3	.004	15	-.755	1
521		14	max	420.285	3	253.642	3	-3.854	15	0	1	.03	1	.235	3
522			min	-88.923	10	-557.062	1	-95.217	1	0	3	.001	15	-.41	1
523		15	max	420.819	3	252.497	3	-3.854	15	0	1	-.001	15	.078	3
524			min	-88.33	10	-558.588	1	-95.217	1	0	3	-.029	1	-.064	1
525		16	max	421.353	3	251.352	3	-3.854	15	0	1	-.004	15	.295	2
526			min	-87.736	10	-560.115	1	-95.217	1	0	3	-.088	1	-.079	3
527		17	max	421.887	3	250.207	3	-3.854	15	0	1	-.006	15	.631	1
528			min	-87.143	10	-561.642	1	-95.217	1	0	3	-.147	1	-.234	3
529		18	max	-8.321	15	528.12	1	-4.53	15	0	3	-.009	15	.315	1
530			min	-201.467	1	-200.761	3	-111.478	1	0	2	-.213	1	-.115	3
531		19	max	-8.106	15	526.593	1	-4.53	15	0	3	-.011	15	.01	3
532			min	-200.755	1	-201.906	3	-111.478	1	0	2	-.282	1	-.012	1
533	M5	1	max	439.146	1	1828.403	3	0	1	0	1	0	1	.028	1
534			min	16.617	12	-1546.209	1	0	1	0	1	0	1	0	15
535		2	max	439.858	1	1827.258	3	0	1	0	1	0	1	.989	1
536			min	16.974	12	-1547.736	1	0	1	0	1	0	1	-1.132	3
537		3	max	1225.716	3	1523.275	1	0	1	0	1	0	1	1.916	1
538			min	-867.983	2	-1242.644	3	0	1	0	1	0	1	-2.232	3
539		4	max	1226.25	3	1521.748	1	0	1	0	1	0	1	.971	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-867.271	2	-1243.79	3	0	1	0	1	0	1	-1.46	3
541		5	max	1226.784	3	1520.221	1	0	1	0	1	0	1	.033	9
542			min	-866.559	2	-1244.935	3	0	1	0	1	0	1	-.688	3
543		6	max	1227.318	3	1518.694	1	0	1	0	1	0	1	.085	3
544			min	-865.846	2	-1246.08	3	0	1	0	1	0	1	-.916	1
545		7	max	1227.852	3	1517.167	1	0	1	0	1	0	1	.859	3
546			min	-865.134	2	-1247.225	3	0	1	0	1	0	1	-1.858	1
547		8	max	1228.386	3	1515.64	1	0	1	0	1	0	1	1.633	3
548			min	-864.422	2	-1248.37	3	0	1	0	1	0	1	-2.799	1
549		9	max	1252.732	3	116.947	2	0	1	0	1	0	1	1.886	3
550			min	-690.697	2	.465	15	0	1	0	1	0	1	-3.173	1
551		10	max	1253.266	3	115.42	2	0	1	0	1	0	1	1.82	3
552			min	-689.985	2	.004	15	0	1	0	1	0	1	-3.213	1
553		11	max	1253.8	3	113.893	2	0	1	0	1	0	1	1.755	3
554			min	-689.273	2	-1.57	4	0	1	0	1	0	1	-3.252	1
555		12	max	1278.343	3	779.513	3	0	1	0	1	0	1	1.537	3
556			min	-515.566	2	-1643.773	1	0	1	0	1	0	1	-2.894	1
557		13	max	1278.877	3	778.368	3	0	1	0	1	0	1	1.053	3
558			min	-514.854	2	-1645.3	1	0	1	0	1	0	1	-1.874	1
559		14	max	1279.411	3	777.223	3	0	1	0	1	0	1	.571	3
560			min	-514.142	2	-1646.827	1	0	1	0	1	0	1	-.852	1
561		15	max	1279.945	3	776.077	3	0	1	0	1	0	1	.233	2
562			min	-513.43	2	-1648.354	1	0	1	0	1	0	1	0	13
563		16	max	1280.479	3	774.932	3	0	1	0	1	0	1	1.202	2
564			min	-512.718	2	-1649.881	1	0	1	0	1	0	1	-.393	3
565		17	max	1281.013	3	773.787	3	0	1	0	1	0	1	2.218	1
566			min	-512.006	2	-1651.408	1	0	1	0	1	0	1	-.873	3
567		18	max	-17.327	12	1797.753	1	0	1	0	1	0	1	1.14	1
568			min	-438.95	1	-700.399	3	0	1	0	1	0	1	-.455	3
569		19	max	-16.971	12	1796.226	1	0	1	0	1	0	1	.025	1
570			min	-438.238	1	-701.544	3	0	1	0	1	0	1	-.02	3
571	M9	1	max	200.301	1	549.684	3	99.464	1	0	3	-.011	15	0	15
572			min	8.091	15	-453.249	1	4.045	15	0	1	-.279	1	-.014	1
573		2	max	201.013	1	548.539	3	99.464	1	0	3	-.009	15	.268	1
574			min	8.306	15	-454.776	1	4.045	15	0	1	-.218	1	-.342	3
575		3	max	386.713	3	523.514	1	98.996	1	0	1	-.006	15	.54	1
576			min	-249.82	2	-397.194	3	4.013	15	0	3	-.156	1	-.672	3
577		4	max	387.247	3	521.987	1	98.996	1	0	1	-.004	15	.215	1
578			min	-249.108	2	-398.339	3	4.013	15	0	3	-.095	1	-.425	3
579		5	max	387.781	3	520.46	1	98.996	1	0	1	-.001	15	-.005	15
580			min	-248.396	2	-399.484	3	4.013	15	0	3	-.033	1	-.177	3
581		6	max	388.315	3	518.933	1	98.996	1	0	1	.028	1	.071	3
582			min	-247.684	2	-400.629	3	4.013	15	0	3	.001	15	-.431	1
583		7	max	388.849	3	517.406	1	98.996	1	0	1	.09	1	.32	3
584			min	-246.972	2	-401.775	3	4.013	15	0	3	.004	15	-.753	1
585		8	max	389.383	3	515.879	1	98.996	1	0	1	.151	1	.57	3
586			min	-246.26	2	-402.92	3	4.013	15	0	3	.006	15	-1.073	1
587		9	max	403.815	3	35.151	2	153.973	1	0	3	-.004	15	.668	3
588			min	-162.27	2	.466	15	6.249	15	0	9	-.094	1	-1.222	1
589		10	max	404.349	3	33.624	2	153.973	1	0	3	.002	1	.649	3
590			min	-161.557	2	.005	15	6.249	15	0	9	0	15	-1.234	1
591		11	max	404.883	3	32.097	2	153.973	1	0	3	.097	1	.632	3
592			min	-160.845	2	-1.828	4	6.249	15	0	9	.004	15	-1.245	1
593		12	max	419.217	3	255.933	3	95.217	1	0	3	-.006	15	.551	3
594			min	-90.11	10	-554.008	1	3.854	15	0	1	-.148	1	-1.1	1
595		13	max	419.751	3	254.787	3	95.217	1	0	3	-.004	15	.393	3
596			min	-89.516	10	-555.535	1	3.854	15	0	1	-.089	1	-.755	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	420.285	3	253.642	3	95.217	1	0	3	-.001	15	.235	3
598		min	-88.923	10	-557.062	1	3.854	15	0	1	-.03	1	-.41	1
599	15	max	420.819	3	252.497	3	95.217	1	0	3	.029	1	.078	3
600		min	-88.33	10	-558.588	1	3.854	15	0	1	.001	15	-.064	1
601	16	max	421.353	3	251.352	3	95.217	1	0	3	.088	1	.295	2
602		min	-87.736	10	-560.115	1	3.854	15	0	1	.004	15	-.079	3
603	17	max	421.887	3	250.207	3	95.217	1	0	3	.147	1	.631	1
604		min	-87.143	10	-561.642	1	3.854	15	0	1	.006	15	-.234	3
605	18	max	-8.321	15	528.12	1	111.478	1	0	2	.213	1	.315	1
606		min	-201.467	1	-200.761	3	4.53	15	0	3	.009	15	-.115	3
607	19	max	-8.106	15	526.593	1	111.478	1	0	2	.282	1	.01	3
608		min	-200.755	1	-201.906	3	4.53	15	0	3	.011	15	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.178	1	.008	3	1.194e-2	1	NC	1	NC	1
2			min	0	15	-.025	3	-.004	2	-1.622e-3	3	NC	1	NC	1
3		2	max	0	1	.218	3	.046	1	1.329e-2	1	NC	5	NC	2
4			min	0	15	-.004	9	.002	10	-1.497e-3	3	962.241	3	5261.34	1
5		3	max	0	1	.416	3	.108	1	1.465e-2	1	NC	5	NC	3
6			min	0	15	-.129	1	.005	15	-1.371e-3	3	531.243	3	2204.251	1
7		4	max	0	1	.536	3	.16	1	1.601e-2	1	NC	5	NC	3
8			min	0	15	-.201	1	.007	15	-1.245e-3	3	417.085	3	1475.121	1
9		5	max	0	1	.566	3	.186	1	1.737e-2	1	NC	5	NC	3
10			min	0	15	-.198	1	.008	15	-1.12e-3	3	396.322	3	1265.328	1
11		6	max	0	1	.506	3	.179	1	1.872e-2	1	NC	5	NC	3
12			min	0	15	-.123	1	.007	15	-9.939e-4	3	440.985	3	1317.869	1
13		7	max	0	1	.375	3	.14	1	2.008e-2	1	NC	5	NC	3
14			min	0	15	-.008	9	.006	15	-8.682e-4	3	584.956	3	1689.935	1
15		8	max	0	1	.209	3	.081	1	2.144e-2	1	NC	1	NC	3
16			min	0	15	.005	15	0	10	-7.426e-4	3	1002.451	3	2959.505	1
17		9	max	0	1	.302	1	.025	3	2.28e-2	1	NC	4	NC	1
18			min	0	15	.009	15	-.007	10	-6.169e-4	3	1887.231	1	NC	1
19		10	max	0	1	.363	1	.023	3	2.415e-2	1	NC	3	NC	1
20			min	0	1	-.011	3	-.016	2	-4.912e-4	3	1262.55	1	NC	1
21		11	max	0	15	.302	1	.025	3	2.28e-2	1	NC	4	NC	1
22			min	0	1	.009	15	-.007	10	-6.169e-4	3	1887.231	1	NC	1
23		12	max	0	15	.209	3	.081	1	2.144e-2	1	NC	1	NC	3
24			min	0	1	.005	15	0	10	-7.426e-4	3	1002.451	3	2959.505	1
25		13	max	0	15	.375	3	.14	1	2.008e-2	1	NC	5	NC	3
26			min	0	1	-.008	9	.006	15	-8.682e-4	3	584.956	3	1689.935	1
27		14	max	0	15	.506	3	.179	1	1.872e-2	1	NC	5	NC	3
28			min	0	1	-.123	1	.007	15	-9.939e-4	3	440.985	3	1317.869	1
29		15	max	0	15	.566	3	.186	1	1.737e-2	1	NC	5	NC	3
30			min	0	1	-.198	1	.008	15	-1.12e-3	3	396.322	3	1265.328	1
31		16	max	0	15	.536	3	.16	1	1.601e-2	1	NC	5	NC	3
32			min	0	1	-.201	1	.007	15	-1.245e-3	3	417.085	3	1475.121	1
33		17	max	0	15	.416	3	.108	1	1.465e-2	1	NC	5	NC	3
34			min	0	1	-.129	1	.005	15	-1.371e-3	3	531.243	3	2204.251	1
35		18	max	0	15	.218	3	.046	1	1.329e-2	1	NC	5	NC	2
36			min	0	1	-.004	9	.002	10	-1.497e-3	3	962.241	3	5261.34	1
37	19	max	0	15	.178	1	.008	3	1.194e-2	1	NC	1	NC	1	
38		min	0	1	-.025	3	-.004	2	-1.622e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.272	3	.007	3	7.245e-3	1	NC	1	NC	1
40			min	0	15	-.554	1	-.003	2	-4.214e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.53	3	.031	1	8.531e-3	1	NC	5	NC	2
42		min	0	15	-.897	1	0	10	-5.058e-3	3	683.504	1	8101.611	1
43	3	max	0	1	.752	3	.084	1	9.817e-3	1	NC	15	NC	3
44		min	0	15	-1.197	1	.004	15	-5.902e-3	3	364.143	1	2841.216	1
45	4	max	0	1	.912	3	.134	1	1.11e-2	1	9360.02	15	NC	3
46		min	0	15	-1.426	1	.006	15	-6.746e-3	3	268.552	1	1765.665	1
47	5	max	0	1	.999	3	.163	1	1.239e-2	1	8073.641	15	NC	3
48		min	0	15	-1.567	1	.007	15	-7.59e-3	3	231.001	1	1453.889	1
49	6	max	0	1	1.011	3	.16	1	1.368e-2	1	7703.398	15	NC	3
50		min	0	15	-1.62	1	.007	15	-8.433e-3	3	219.554	1	1475.096	1
51	7	max	0	1	.96	3	.128	1	1.496e-2	1	7921.422	15	NC	3
52		min	0	15	-1.596	1	.005	15	-9.277e-3	3	224.59	1	1856.269	1
53	8	max	0	1	.871	3	.075	1	1.625e-2	1	8589.309	15	NC	2
54		min	0	15	-1.522	1	0	10	-1.012e-2	3	241.928	1	3198.667	1
55	9	max	0	1	.781	3	.022	3	1.753e-2	1	9488.943	15	NC	1
56		min	0	15	-1.436	1	-.006	10	-1.096e-2	3	265.431	1	NC	1
57	10	max	0	1	.737	3	.021	3	1.882e-2	1	NC	15	NC	1
58		min	0	1	-1.393	1	-.014	2	-1.181e-2	3	279.039	1	NC	1
59	11	max	0	15	.781	3	.022	3	1.753e-2	1	9488.943	15	NC	1
60		min	0	1	-1.436	1	-.006	10	-1.096e-2	3	265.431	1	NC	1
61	12	max	0	15	.871	3	.075	1	1.625e-2	1	8589.309	15	NC	2
62		min	0	1	-1.522	1	0	10	-1.012e-2	3	241.928	1	3198.667	1
63	13	max	0	15	.96	3	.128	1	1.496e-2	1	7921.422	15	NC	3
64		min	0	1	-1.596	1	.005	15	-9.277e-3	3	224.59	1	1856.269	1
65	14	max	0	15	1.011	3	.16	1	1.368e-2	1	7703.398	15	NC	3
66		min	0	1	-1.62	1	.007	15	-8.433e-3	3	219.554	1	1475.096	1
67	15	max	0	15	.999	3	.163	1	1.239e-2	1	8073.641	15	NC	3
68		min	0	1	-1.567	1	.007	15	-7.59e-3	3	231.001	1	1453.889	1
69	16	max	0	15	.912	3	.134	1	1.11e-2	1	9360.02	15	NC	3
70		min	0	1	-1.426	1	.006	15	-6.746e-3	3	268.552	1	1765.665	1
71	17	max	0	15	.752	3	.084	1	9.817e-3	1	NC	15	NC	3
72		min	0	1	-1.197	1	.004	15	-5.902e-3	3	364.143	1	2841.216	1
73	18	max	0	15	.53	3	.031	1	8.531e-3	1	NC	5	NC	2
74		min	0	1	-.897	1	0	10	-5.058e-3	3	683.504	1	8101.611	1
75	19	max	0	15	.272	3	.007	3	7.245e-3	1	NC	1	NC	1
76		min	0	1	-.554	1	-.003	2	-4.214e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.279	.006	3	3.496e-3	3	NC	1	NC	1
78		min	0	1	-.554	1	-.003	2	-7.397e-3	1	NC	1	NC	1
79	2	max	0	15	.449	3	.031	1	4.196e-3	3	NC	5	NC	2
80		min	0	1	-.926	1	0	10	-8.72e-3	1	628.901	1	8031.679	1
81	3	max	0	15	.6	3	.085	1	4.895e-3	3	NC	15	NC	3
82		min	0	1	-1.25	1	.004	15	-1.004e-2	1	336.037	1	2826.786	1
83	4	max	0	15	.718	3	.135	1	5.595e-3	3	9373.932	15	NC	3
84		min	0	1	-1.493	1	.006	15	-1.137e-2	1	249.047	1	1758.751	1
85	5	max	0	15	.796	3	.163	1	6.294e-3	3	8087.032	15	NC	3
86		min	0	1	-1.638	1	.007	15	-1.269e-2	1	215.786	1	1448.783	1
87	6	max	0	15	.832	3	.161	1	6.994e-3	3	7718.037	15	NC	3
88		min	0	1	-1.683	1	.007	15	-1.401e-2	1	207.198	1	1469.784	1
89	7	max	0	15	.832	3	.128	1	7.694e-3	3	7939.066	15	NC	3
90		min	0	1	-1.642	1	.005	15	-1.534e-2	1	214.908	1	1848.155	1
91	8	max	0	15	.807	3	.075	1	8.393e-3	3	8612.003	15	NC	2
92		min	0	1	-1.547	1	.001	10	-1.666e-2	1	235.613	1	3176.018	1
93	9	max	0	15	.774	3	.022	1	9.093e-3	3	9518.163	15	NC	1
94		min	0	1	-1.442	1	-.005	10	-1.798e-2	1	263.37	1	NC	1
95	10	max	0	1	.756	3	.019	3	9.793e-3	3	NC	15	NC	1
96		min	0	1	-1.39	1	-.013	2	-1.931e-2	1	279.606	1	NC	1
97	11	max	0	1	.774	3	.022	1	9.093e-3	3	9518.163	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.442	1	-.005	10	-1.798e-2	1	263.37	1	NC	1
99		max	0	1	.807	3	.075	1	8.393e-3	3	8612.003	15	NC	2
100		min	0	15	-1.547	1	.001	10	-1.666e-2	1	235.613	1	3176.018	1
101		max	0	1	.832	3	.128	1	7.694e-3	3	7939.066	15	NC	3
102		min	0	15	-1.642	1	.005	15	-1.534e-2	1	214.908	1	1848.155	1
103		max	0	1	.832	3	.161	1	6.994e-3	3	7718.037	15	NC	3
104		min	0	15	-1.683	1	.007	15	-1.401e-2	1	207.198	1	1469.784	1
105		max	0	1	.796	3	.163	1	6.294e-3	3	8087.032	15	NC	3
106		min	0	15	-1.638	1	.007	15	-1.269e-2	1	215.786	1	1448.783	1
107		max	0	1	.718	3	.135	1	5.595e-3	3	9373.932	15	NC	3
108		min	0	15	-1.493	1	.006	15	-1.137e-2	1	249.047	1	1758.751	1
109		max	0	1	.6	3	.085	1	4.895e-3	3	NC	15	NC	3
110		min	0	15	-1.25	1	.004	15	-1.004e-2	1	336.037	1	2826.786	1
111		max	0	1	.449	3	.031	1	4.196e-3	3	NC	5	NC	2
112		min	0	15	-.926	1	0	10	-8.72e-3	1	628.901	1	8031.679	1
113		max	0	1	.279	3	.006	3	3.496e-3	3	NC	1	NC	1
114		min	0	15	-.554	1	-.003	2	-7.397e-3	1	NC	1	NC	1
115	M16	max	0	15	.173	1	.005	3	6.421e-3	3	NC	1	NC	1
116		min	-.001	1	-.096	3	-.003	2	-1.118e-2	1	NC	1	NC	1
117		max	0	15	.005	4	.045	1	7.371e-3	3	NC	5	NC	2
118		min	0	1	-.057	2	.002	15	-1.235e-2	1	1135.763	2	5329.297	1
119		max	0	15	.026	3	.107	1	8.322e-3	3	NC	5	NC	3
120		min	0	1	-.22	2	.004	15	-1.353e-2	1	634.306	2	2219.174	1
121		max	0	15	.05	3	.16	1	9.272e-3	3	NC	5	NC	3
122		min	0	1	-.31	2	.007	15	-1.471e-2	1	508.872	2	1480.109	1
123		max	0	15	.042	3	.186	1	1.022e-2	3	NC	5	NC	3
124		min	0	1	-.316	2	.008	15	-1.588e-2	1	502.469	2	1266.174	1
125		max	0	15	.004	12	.179	1	1.117e-2	3	NC	5	NC	3
126		min	0	1	-.24	2	.007	15	-1.706e-2	1	601.515	2	1314.726	1
127		max	0	15	.005	4	.141	1	1.212e-2	3	NC	5	NC	3
128		min	0	1	-.099	2	.006	15	-1.823e-2	1	943.583	2	1677.633	1
129		max	0	15	.123	1	.082	1	1.307e-2	3	NC	4	NC	3
130		min	0	1	-.135	3	.003	10	-1.941e-2	1	3059.696	2	2902.434	1
131		max	0	15	.279	1	.024	1	1.403e-2	3	NC	5	NC	1
132		min	0	1	-.2	3	-.004	10	-2.059e-2	1	2200.214	1	NC	1
133		max	0	1	.349	1	.017	3	1.498e-2	3	NC	5	NC	1
134		min	0	1	-.228	3	-.012	2	-2.176e-2	1	1328.76	1	NC	1
135		max	0	1	.279	1	.024	1	1.403e-2	3	NC	5	NC	1
136		min	0	15	-.2	3	-.004	10	-2.059e-2	1	2200.214	1	NC	1
137		max	0	1	.123	1	.082	1	1.307e-2	3	NC	4	NC	3
138		min	0	15	-.135	3	.003	10	-1.941e-2	1	3059.696	2	2902.434	1
139		max	0	1	.005	4	.141	1	1.212e-2	3	NC	5	NC	3
140		min	0	15	-.099	2	.006	15	-1.823e-2	1	943.583	2	1677.633	1
141		max	0	1	.004	12	.179	1	1.117e-2	3	NC	5	NC	3
142		min	0	15	-.24	2	.007	15	-1.706e-2	1	601.515	2	1314.726	1
143		max	0	1	.042	3	.186	1	1.022e-2	3	NC	5	NC	3
144		min	0	15	-.316	2	.008	15	-1.588e-2	1	502.469	2	1266.174	1
145		max	0	1	.05	3	.16	1	9.272e-3	3	NC	5	NC	3
146		min	0	15	-.31	2	.007	15	-1.471e-2	1	508.872	2	1480.109	1
147		max	0	1	.026	3	.107	1	8.322e-3	3	NC	5	NC	3
148		min	0	15	-.22	2	.004	15	-1.353e-2	1	634.306	2	2219.174	1
149		max	0	1	.005	4	.045	1	7.371e-3	3	NC	5	NC	2
150		min	0	15	-.057	2	.002	15	-1.235e-2	1	1135.763	2	5329.297	1
151		max	.001	1	.173	1	.005	3	6.421e-3	3	NC	1	NC	1
152		min	0	15	-.096	3	-.003	2	-1.118e-2	1	NC	1	NC	1
153	M2	max	.007	1	.006	2	.013	1	-1.218e-5	15	NC	1	NC	2
154		min	-.007	3	-.011	3	0	15	-3.007e-4	1	NC	1	5304.81	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.007	1	.005	2	.012	1	-1.149e-5	15	NC	1	NC	2
156		min	-.006	3	-.011	3	0	15	-2.837e-4	1	NC	1	5784.098	1
157	3	max	.006	1	.004	2	.011	1	-1.08e-5	15	NC	1	NC	2
158		min	-.006	3	-.01	3	0	15	-2.667e-4	1	NC	1	6354.903	1
159	4	max	.006	1	.003	2	.01	1	-1.011e-5	15	NC	1	NC	2
160		min	-.006	3	-.01	3	0	15	-2.497e-4	1	NC	1	7041.382	1
161	5	max	.005	1	.002	2	.009	1	-9.425e-6	15	NC	1	NC	2
162		min	-.005	3	-.01	3	0	15	-2.327e-4	1	NC	1	7876.461	1
163	6	max	.005	1	.001	2	.008	1	-8.736e-6	15	NC	1	NC	2
164		min	-.005	3	-.01	3	0	15	-2.157e-4	1	NC	1	8905.974	1
165	7	max	.005	1	0	2	.007	1	-8.048e-6	15	NC	1	NC	1
166		min	-.004	3	-.009	3	0	15	-1.986e-4	1	NC	1	NC	1
167	8	max	.004	1	0	2	.006	1	-7.36e-6	15	NC	1	NC	1
168		min	-.004	3	-.009	3	0	15	-1.816e-4	1	NC	1	NC	1
169	9	max	.004	1	-.001	2	.005	1	-6.672e-6	15	NC	1	NC	1
170		min	-.004	3	-.008	3	0	15	-1.646e-4	1	NC	1	NC	1
171	10	max	.004	1	-.001	15	.004	1	-5.984e-6	15	NC	1	NC	1
172		min	-.003	3	-.008	3	0	15	-1.476e-4	1	NC	1	NC	1
173	11	max	.003	1	-.001	15	.003	1	-5.296e-6	15	NC	1	NC	1
174		min	-.003	3	-.007	3	0	15	-1.306e-4	1	NC	1	NC	1
175	12	max	.003	1	-.001	15	.003	1	-4.607e-6	15	NC	1	NC	1
176		min	-.003	3	-.007	3	0	15	-1.136e-4	1	NC	1	NC	1
177	13	max	.002	1	-.001	15	.002	1	-3.919e-6	15	NC	1	NC	1
178		min	-.002	3	-.006	3	0	15	-9.655e-5	1	NC	1	NC	1
179	14	max	.002	1	-.001	15	.001	1	-3.231e-6	15	NC	1	NC	1
180		min	-.002	3	-.005	3	0	15	-7.954e-5	1	NC	1	NC	1
181	15	max	.002	1	-.001	15	0	1	-2.543e-6	15	NC	1	NC	1
182		min	-.001	3	-.005	4	0	15	-6.252e-5	1	NC	1	NC	1
183	16	max	.001	1	0	15	0	1	-1.855e-6	15	NC	1	NC	1
184		min	-.001	3	-.004	4	0	15	-4.551e-5	1	NC	1	NC	1
185	17	max	0	1	0	15	0	1	-1.167e-6	15	NC	1	NC	1
186		min	0	3	-.003	4	0	15	-2.849e-5	1	NC	1	NC	1
187	18	max	0	1	0	15	0	1	-4.785e-7	15	NC	1	NC	1
188		min	0	3	-.002	4	0	15	-1.147e-5	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	5.54e-6	1	NC	1	NC	1
190		min	0	1	0	1	0	1	7.241e-8	12	NC	1	NC	1
191	M3	1	max	0	0	1	0	1	-1.172e-7	12	NC	1	NC	1
192		min	0	1	0	1	0	1	-3.273e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	3.216e-5	1	NC	1	NC	1
194		min	0	2	-.003	4	0	12	1.302e-6	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	6.76e-5	1	NC	1	NC	1
196		min	0	2	-.006	4	0	12	2.734e-6	15	NC	1	NC	1
197	4	max	0	3	-.002	15	0	1	1.03e-4	1	NC	1	NC	1
198		min	0	2	-.009	4	0	12	4.165e-6	15	NC	1	NC	1
199	5	max	.001	3	-.003	15	0	1	1.385e-4	1	NC	1	NC	1
200		min	0	2	-.012	4	0	12	5.597e-6	15	8792.805	4	NC	1
201	6	max	.002	3	-.003	15	0	1	1.739e-4	1	NC	2	NC	1
202		min	-.001	2	-.015	4	0	15	7.028e-6	15	7098.313	4	NC	1
203	7	max	.002	3	-.004	15	0	1	2.093e-4	1	NC	5	NC	1
204		min	-.001	2	-.017	4	0	15	8.46e-6	15	6079.122	4	NC	1
205	8	max	.002	3	-.004	15	.001	1	2.448e-4	1	NC	5	NC	1
206		min	-.002	2	-.019	4	0	15	9.891e-6	15	5450.191	4	NC	1
207	9	max	.003	3	-.005	15	.002	1	2.802e-4	1	NC	5	NC	1
208		min	-.002	2	-.02	4	0	15	1.132e-5	15	5077.449	4	NC	1
209	10	max	.003	3	-.005	15	.002	1	3.157e-4	1	NC	5	NC	1
210		min	-.002	2	-.021	4	0	15	1.275e-5	15	4895.736	4	NC	1
211	11	max	.003	3	-.005	15	.003	1	3.511e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.002	2	-.021	4	0	15	1.419e-5	15	4878.202	4	NC	1
213		max	.004	3	-.005	15	.003	1	3.865e-4	1	NC	5	NC	1
214		min	-.003	2	-.021	4	0	15	1.562e-5	15	5026.009	4	NC	1
215		max	.004	3	-.005	15	.004	1	4.22e-4	1	NC	5	NC	1
216		min	-.003	2	-.02	4	0	15	1.705e-5	15	5370.08	4	NC	1
217		max	.004	3	-.004	15	.005	1	4.574e-4	1	NC	5	NC	1
218		min	-.003	2	-.018	4	0	15	1.848e-5	15	5987.22	4	NC	1
219		max	.005	3	-.004	15	.006	1	4.928e-4	1	NC	3	NC	1
220		min	-.003	2	-.015	4	0	15	1.991e-5	15	7047.997	4	NC	1
221		max	.005	3	-.003	15	.007	1	5.283e-4	1	NC	1	NC	1
222		min	-.004	2	-.012	4	0	15	2.134e-5	15	8965.765	4	NC	1
223		max	.005	3	-.002	15	.009	1	5.637e-4	1	NC	1	NC	1
224		min	-.004	2	-.009	4	0	15	2.277e-5	15	NC	1	NC	1
225		max	.006	3	-.001	15	.01	1	5.991e-4	1	NC	1	NC	2
226		min	-.004	2	-.006	1	0	15	2.421e-5	15	NC	1	9831.695	1
227		max	.006	3	0	15	.012	1	6.346e-4	1	NC	1	NC	2
228		min	-.004	2	-.003	1	0	15	2.564e-5	15	NC	1	8428.607	1
229	M4	max	.003	1	.004	2	0	15	1.568e-4	1	NC	1	NC	3
230		min	0	3	-.006	3	-.012	1	6.364e-6	15	NC	1	2049.595	1
231		max	.003	1	.004	2	0	15	1.568e-4	1	NC	1	NC	3
232		min	0	3	-.006	3	-.011	1	6.364e-6	15	NC	1	2227.057	1
233		max	.002	1	.004	2	0	15	1.568e-4	1	NC	1	NC	3
234		min	0	3	-.005	3	-.01	1	6.364e-6	15	NC	1	2438.367	1
235		max	.002	1	.003	2	0	15	1.568e-4	1	NC	1	NC	3
236		min	0	3	-.005	3	-.009	1	6.364e-6	15	NC	1	2692.301	1
237		max	.002	1	.003	2	0	15	1.568e-4	1	NC	1	NC	3
238		min	0	3	-.005	3	-.008	1	6.364e-6	15	NC	1	3000.783	1
239		max	.002	1	.003	2	0	15	1.568e-4	1	NC	1	NC	3
240		min	0	3	-.004	3	-.007	1	6.364e-6	15	NC	1	3380.353	1
241		max	.002	1	.003	2	0	15	1.568e-4	1	NC	1	NC	3
242		min	0	3	-.004	3	-.006	1	6.364e-6	15	NC	1	3854.487	1
243		max	.002	1	.002	2	0	15	1.568e-4	1	NC	1	NC	2
244		min	0	3	-.004	3	-.006	1	6.364e-6	15	NC	1	4457.42	1
245		max	.002	1	.002	2	0	15	1.568e-4	1	NC	1	NC	2
246		min	0	3	-.003	3	-.005	1	6.364e-6	15	NC	1	5240.69	1
247		max	.001	1	.002	2	0	15	1.568e-4	1	NC	1	NC	2
248		min	0	3	-.003	3	-.004	1	6.364e-6	15	NC	1	6284.853	1
249		max	.001	1	.002	2	0	15	1.568e-4	1	NC	1	NC	2
250		min	0	3	-.003	3	-.003	1	6.364e-6	15	NC	1	7721.673	1
251		max	.001	1	.002	2	0	15	1.568e-4	1	NC	1	NC	2
252		min	0	3	-.002	3	-.003	1	6.364e-6	15	NC	1	9779.047	1
253		max	0	1	.001	2	0	15	1.568e-4	1	NC	1	NC	1
254		min	0	3	-.002	3	-.002	1	6.364e-6	15	NC	1	NC	1
255		max	0	1	.001	2	0	15	1.568e-4	1	NC	1	NC	1
256		min	0	3	-.002	3	-.001	1	6.364e-6	15	NC	1	NC	1
257		max	0	1	0	2	0	15	1.568e-4	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	6.364e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	1.568e-4	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	6.364e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	1.568e-4	1	NC	1	NC	1
262		min	0	3	0	3	0	1	6.364e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	1.568e-4	1	NC	1	NC	1
264		min	0	3	0	3	0	1	6.364e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	1.568e-4	1	NC	1	NC	1
266		min	0	1	0	1	0	1	6.364e-6	15	NC	1	NC	1
267	M6	max	.022	1	.025	2	0	1	0	1	NC	3	NC	1
268		min	-.022	3	-.034	3	0	1	0	1	2774.552	2	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.021	1	.023	2	0	1	0	1	NC	3	NC	1
270		min	-.021	3	-.032	3	0	1	0	1	3060.48	2	NC	1
271	3	max	.02	1	.02	2	0	1	0	1	NC	3	NC	1
272		min	-.019	3	-.03	3	0	1	0	1	3409.002	2	NC	1
273	4	max	.019	1	.018	2	0	1	0	1	NC	3	NC	1
274		min	-.018	3	-.029	3	0	1	0	1	3839.256	2	NC	1
275	5	max	.017	1	.016	2	0	1	0	1	NC	3	NC	1
276		min	-.017	3	-.027	3	0	1	0	1	4378.567	2	NC	1
277	6	max	.016	1	.014	2	0	1	0	1	NC	3	NC	1
278		min	-.016	3	-.025	3	0	1	0	1	5067.08	2	NC	1
279	7	max	.015	1	.012	2	0	1	0	1	NC	1	NC	1
280		min	-.015	3	-.023	3	0	1	0	1	5965.803	2	NC	1
281	8	max	.014	1	.01	2	0	1	0	1	NC	1	NC	1
282		min	-.013	3	-.021	3	0	1	0	1	7171.316	2	NC	1
283	9	max	.012	1	.008	2	0	1	0	1	NC	1	NC	1
284		min	-.012	3	-.02	3	0	1	0	1	8844.383	2	NC	1
285	10	max	.011	1	.006	2	0	1	0	1	NC	1	NC	1
286		min	-.011	3	-.018	3	0	1	0	1	NC	1	NC	1
287	11	max	.01	1	.005	2	0	1	0	1	NC	1	NC	1
288		min	-.01	3	-.016	3	0	1	0	1	NC	1	NC	1
289	12	max	.009	1	.003	2	0	1	0	1	NC	1	NC	1
290		min	-.008	3	-.014	3	0	1	0	1	NC	1	NC	1
291	13	max	.007	1	.002	2	0	1	0	1	NC	1	NC	1
292		min	-.007	3	-.012	3	0	1	0	1	NC	1	NC	1
293	14	max	.006	1	.001	2	0	1	0	1	NC	1	NC	1
294		min	-.006	3	-.01	3	0	1	0	1	NC	1	NC	1
295	15	max	.005	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
297	16	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.002	3	-.004	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	0	1	0	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.009	3	0	1	0	1	NC	1	NC	1
313	5	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.012	3	0	1	0	1	9034.784	4	NC	1
315	6	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.014	4	0	1	0	1	7275.189	4	NC	1
317	7	max	.006	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.006	2	-.017	4	0	1	0	1	6217.904	4	NC	1
319	8	max	.007	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.007	2	-.019	4	0	1	0	1	5565.32	4	NC	1
321	9	max	.008	3	-.005	15	0	1	0	1	NC	5	NC	1
322		min	-.008	2	-.02	4	0	1	0	1	5177.544	4	NC	1
323	10	max	.009	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.009	2	-.021	4	0	1	0	1	4986.488	4	NC	1
325	11	max	.01	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.01	2	-.021	4	0	1	0	1	4963.814	4	NC	1
327		12	max	.011	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.011	2	-.021	4	0	1	0	1	5110.047	4	NC	1
329		13	max	.012	3	-.005	15	0	1	0	1	NC	5	NC	1
330			min	-.012	2	-.02	4	0	1	0	1	5456.141	4	NC	1
331		14	max	.013	3	-.004	15	0	1	0	1	NC	5	NC	1
332			min	-.013	2	-.018	4	0	1	0	1	6079.721	4	NC	1
333		15	max	.014	3	-.004	15	0	1	0	1	NC	1	NC	1
334			min	-.013	2	-.015	4	0	1	0	1	7153.58	4	NC	1
335		16	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.014	2	-.013	4	0	1	0	1	9096.774	4	NC	1
337		17	max	.016	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.015	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.017	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.016	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.018	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.017	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.016	2	0	1	0	1	NC	1	NC	1
344			min	0	3	-.019	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.016	2	0	1	0	1	NC	1	NC	1
346			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	1	.015	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.014	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	1	.012	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.011	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.012	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.009	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.007	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	1	.006	2	0	15	3.007e-4	1	NC	1	NC	2
382			min	-.007	3	-.011	3	-.013	1	1.218e-5	15	NC	1	5304.81	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	1	.005	2	0	15	2.837e-4	1	NC	1	NC	2
384		min	-.006	3	-.011	3	-.012	1	1.149e-5	15	NC	1	5784.098	1
385	3	max	.006	1	.004	2	0	15	2.667e-4	1	NC	1	NC	2
386		min	-.006	3	-.01	3	-.011	1	1.08e-5	15	NC	1	6354.903	1
387	4	max	.006	1	.003	2	0	15	2.497e-4	1	NC	1	NC	2
388		min	-.006	3	-.01	3	-.01	1	1.011e-5	15	NC	1	7041.382	1
389	5	max	.005	1	.002	2	0	15	2.327e-4	1	NC	1	NC	2
390		min	-.005	3	-.01	3	-.009	1	9.425e-6	15	NC	1	7876.461	1
391	6	max	.005	1	.001	2	0	15	2.157e-4	1	NC	1	NC	2
392		min	-.005	3	-.01	3	-.008	1	8.736e-6	15	NC	1	8905.974	1
393	7	max	.005	1	0	2	0	15	1.986e-4	1	NC	1	NC	1
394		min	-.004	3	-.009	3	-.007	1	8.048e-6	15	NC	1	NC	1
395	8	max	.004	1	0	2	0	15	1.816e-4	1	NC	1	NC	1
396		min	-.004	3	-.009	3	-.006	1	7.36e-6	15	NC	1	NC	1
397	9	max	.004	1	-.001	2	0	15	1.646e-4	1	NC	1	NC	1
398		min	-.004	3	-.008	3	-.005	1	6.672e-6	15	NC	1	NC	1
399	10	max	.004	1	-.001	15	0	15	1.476e-4	1	NC	1	NC	1
400		min	-.003	3	-.008	3	-.004	1	5.984e-6	15	NC	1	NC	1
401	11	max	.003	1	-.001	15	0	15	1.306e-4	1	NC	1	NC	1
402		min	-.003	3	-.007	3	-.003	1	5.296e-6	15	NC	1	NC	1
403	12	max	.003	1	-.001	15	0	15	1.136e-4	1	NC	1	NC	1
404		min	-.003	3	-.007	3	-.003	1	4.607e-6	15	NC	1	NC	1
405	13	max	.002	1	-.001	15	0	15	9.655e-5	1	NC	1	NC	1
406		min	-.002	3	-.006	3	-.002	1	3.919e-6	15	NC	1	NC	1
407	14	max	.002	1	-.001	15	0	15	7.954e-5	1	NC	1	NC	1
408		min	-.002	3	-.005	3	-.001	1	3.231e-6	15	NC	1	NC	1
409	15	max	.002	1	-.001	15	0	15	6.252e-5	1	NC	1	NC	1
410		min	-.001	3	-.005	4	0	1	2.543e-6	15	NC	1	NC	1
411	16	max	.001	1	0	15	0	15	4.551e-5	1	NC	1	NC	1
412		min	-.001	3	-.004	4	0	1	1.855e-6	15	NC	1	NC	1
413	17	max	0	1	0	15	0	15	2.849e-5	1	NC	1	NC	1
414		min	0	3	-.003	4	0	1	1.167e-6	15	NC	1	NC	1
415	18	max	0	1	0	15	0	15	1.147e-5	1	NC	1	NC	1
416		min	0	3	-.002	4	0	1	4.785e-7	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	-7.241e-8	12	NC	1	NC	1
418		min	0	1	0	1	0	1	-5.54e-6	1	NC	1	NC	1
419	M11	1	max	0	0	1	0	1	3.273e-6	1	NC	1	NC	1
420		min	0	1	0	1	0	1	1.172e-7	12	NC	1	NC	1
421	2	max	0	3	0	15	0	12	-1.302e-6	15	NC	1	NC	1
422		min	0	2	-.003	4	0	1	-3.216e-5	1	NC	1	NC	1
423	3	max	0	3	-.001	15	0	12	-2.734e-6	15	NC	1	NC	1
424		min	0	2	-.006	4	0	1	-6.76e-5	1	NC	1	NC	1
425	4	max	0	3	-.002	15	0	12	-4.165e-6	15	NC	1	NC	1
426		min	0	2	-.009	4	0	1	-1.03e-4	1	NC	1	NC	1
427	5	max	.001	3	-.003	15	0	12	-5.597e-6	15	NC	1	NC	1
428		min	0	2	-.012	4	0	1	-1.385e-4	1	8792.805	4	NC	1
429	6	max	.002	3	-.003	15	0	15	-7.028e-6	15	NC	2	NC	1
430		min	-.001	2	-.015	4	0	1	-1.739e-4	1	7098.313	4	NC	1
431	7	max	.002	3	-.004	15	0	15	-8.46e-6	15	NC	5	NC	1
432		min	-.001	2	-.017	4	0	1	-2.093e-4	1	6079.122	4	NC	1
433	8	max	.002	3	-.004	15	0	15	-9.891e-6	15	NC	5	NC	1
434		min	-.002	2	-.019	4	-.001	1	-2.448e-4	1	5450.191	4	NC	1
435	9	max	.003	3	-.005	15	0	15	-1.132e-5	15	NC	5	NC	1
436		min	-.002	2	-.02	4	-.002	1	-2.802e-4	1	5077.449	4	NC	1
437	10	max	.003	3	-.005	15	0	15	-1.275e-5	15	NC	5	NC	1
438		min	-.002	2	-.021	4	-.002	1	-3.157e-4	1	4895.736	4	NC	1
439	11	max	.003	3	-.005	15	0	15	-1.419e-5	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.002	2	-.021	4	-.003	1	-3.511e-4	1	4878.202	4	NC	1
441		max	.004	3	-.005	15	0	15	-1.562e-5	15	NC	5	NC	1
442		min	-.003	2	-.021	4	-.003	1	-3.865e-4	1	5026.009	4	NC	1
443		max	.004	3	-.005	15	0	15	-1.705e-5	15	NC	5	NC	1
444		min	-.003	2	-.02	4	-.004	1	-4.22e-4	1	5370.08	4	NC	1
445		max	.004	3	-.004	15	0	15	-1.848e-5	15	NC	5	NC	1
446		min	-.003	2	-.018	4	-.005	1	-4.574e-4	1	5987.22	4	NC	1
447		max	.005	3	-.004	15	0	15	-1.991e-5	15	NC	3	NC	1
448		min	-.003	2	-.015	4	-.006	1	-4.928e-4	1	7047.997	4	NC	1
449		max	.005	3	-.003	15	0	15	-2.134e-5	15	NC	1	NC	1
450		min	-.004	2	-.012	4	-.007	1	-5.283e-4	1	8965.765	4	NC	1
451		max	.005	3	-.002	15	0	15	-2.277e-5	15	NC	1	NC	1
452		min	-.004	2	-.009	4	-.009	1	-5.637e-4	1	NC	1	NC	1
453		max	.006	3	-.001	15	0	15	-2.421e-5	15	NC	1	NC	2
454		min	-.004	2	-.006	1	-.01	1	-5.991e-4	1	NC	1	9831.695	1
455		max	.006	3	0	15	0	15	-2.564e-5	15	NC	1	NC	2
456		min	-.004	2	-.003	1	-.012	1	-6.346e-4	1	NC	1	8428.607	1
457	M12	max	.003	1	.004	2	.012	1	-6.364e-6	15	NC	1	NC	3
458		min	0	3	-.006	3	0	15	-1.568e-4	1	NC	1	2049.595	1
459		max	.003	1	.004	2	.011	1	-6.364e-6	15	NC	1	NC	3
460		min	0	3	-.006	3	0	15	-1.568e-4	1	NC	1	2227.057	1
461		max	.002	1	.004	2	.01	1	-6.364e-6	15	NC	1	NC	3
462		min	0	3	-.005	3	0	15	-1.568e-4	1	NC	1	2438.367	1
463		max	.002	1	.003	2	.009	1	-6.364e-6	15	NC	1	NC	3
464		min	0	3	-.005	3	0	15	-1.568e-4	1	NC	1	2692.301	1
465		max	.002	1	.003	2	.008	1	-6.364e-6	15	NC	1	NC	3
466		min	0	3	-.005	3	0	15	-1.568e-4	1	NC	1	3000.783	1
467		max	.002	1	.003	2	.007	1	-6.364e-6	15	NC	1	NC	3
468		min	0	3	-.004	3	0	15	-1.568e-4	1	NC	1	3380.353	1
469		max	.002	1	.003	2	.006	1	-6.364e-6	15	NC	1	NC	3
470		min	0	3	-.004	3	0	15	-1.568e-4	1	NC	1	3854.487	1
471		max	.002	1	.002	2	.006	1	-6.364e-6	15	NC	1	NC	2
472		min	0	3	-.004	3	0	15	-1.568e-4	1	NC	1	4457.42	1
473		max	.002	1	.002	2	.005	1	-6.364e-6	15	NC	1	NC	2
474		min	0	3	-.003	3	0	15	-1.568e-4	1	NC	1	5240.69	1
475		max	.001	1	.002	2	.004	1	-6.364e-6	15	NC	1	NC	2
476		min	0	3	-.003	3	0	15	-1.568e-4	1	NC	1	6284.853	1
477		max	.001	1	.002	2	.003	1	-6.364e-6	15	NC	1	NC	2
478		min	0	3	-.003	3	0	15	-1.568e-4	1	NC	1	7721.673	1
479		max	.001	1	.002	2	.003	1	-6.364e-6	15	NC	1	NC	2
480		min	0	3	-.002	3	0	15	-1.568e-4	1	NC	1	9779.047	1
481		max	0	1	.001	2	.002	1	-6.364e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-1.568e-4	1	NC	1	NC	1
483		max	0	1	.001	2	.001	1	-6.364e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-1.568e-4	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-6.364e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-1.568e-4	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-6.364e-6	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-1.568e-4	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-6.364e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-1.568e-4	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-6.364e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-1.568e-4	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-6.364e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-1.568e-4	1	NC	1	NC	1
495	M1	max	.008	3	.178	1	0	1	1.384e-2	1	NC	1	NC	1
496		min	-.004	2	-.025	3	0	15	-1.94e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.008	3	.088	1	0	15	6.673e-3	1	NC	5	NC	1
498			min	-.004	2	-.012	3	-.009	1	-9.631e-3	3	1502.184	1	NC	1
499		3	max	.008	3	.011	3	0	15	-6.608e-7	10	NC	5	NC	2
500			min	-.004	2	-.01	2	-.013	1	-2.83e-4	1	721.125	1	9748.617	1
501		4	max	.007	3	.05	3	0	15	4.761e-3	1	NC	15	NC	1
502			min	-.004	2	-.119	1	-.012	1	-3.858e-3	3	453.197	1	NC	1
503		5	max	.007	3	.102	3	0	15	9.805e-3	1	9546.857	15	NC	1
504			min	-.004	2	-.235	1	-.008	1	-7.619e-3	3	325.654	1	NC	1
505		6	max	.007	3	.158	3	0	15	1.485e-2	1	7536.806	15	NC	1
506			min	-.003	2	-.348	1	-.004	1	-1.138e-2	3	255.583	1	NC	1
507		7	max	.007	3	.212	3	0	1	1.989e-2	1	6350.97	15	NC	1
508			min	-.003	2	-.45	1	0	3	-1.514e-2	3	214.332	1	NC	1
509		8	max	.007	3	.257	3	.001	1	2.494e-2	1	5649.728	15	NC	1
510			min	-.003	2	-.53	1	0	15	-1.89e-2	3	189.983	1	NC	1
511		9	max	.007	3	.287	3	0	15	2.74e-2	1	5283.179	15	NC	1
512			min	-.003	2	-.581	1	0	1	-1.909e-2	3	177.306	1	NC	1
513		10	max	.007	3	.298	3	0	1	2.815e-2	1	5171.239	15	NC	1
514			min	-.003	2	-.598	1	0	15	-1.689e-2	3	173.502	1	NC	1
515		11	max	.006	3	.291	3	0	1	2.889e-2	1	5282.975	15	NC	1
516			min	-.003	2	-.58	1	0	15	-1.469e-2	3	177.531	1	NC	1
517		12	max	.006	3	.267	3	0	15	2.721e-2	1	5649.302	15	NC	1
518			min	-.003	2	-.529	1	-.001	1	-1.238e-2	3	190.674	1	NC	1
519		13	max	.006	3	.227	3	0	15	2.193e-2	1	6350.234	15	NC	1
520			min	-.003	2	-.446	1	0	1	-9.902e-3	3	216.023	1	NC	1
521		14	max	.006	3	.177	3	.003	1	1.664e-2	1	7535.572	15	NC	1
522			min	-.003	2	-.343	1	0	15	-7.424e-3	3	259.199	1	NC	1
523		15	max	.006	3	.12	3	.008	1	1.136e-2	1	9544.739	15	NC	1
524			min	-.003	2	-.229	1	0	15	-4.947e-3	3	333.07	1	NC	1
525		16	max	.006	3	.061	3	.011	1	6.073e-3	1	NC	15	NC	1
526			min	-.003	2	-.113	1	0	15	-2.47e-3	3	468.702	1	NC	1
527		17	max	.005	3	.004	3	.012	1	7.879e-4	1	NC	5	NC	1
528			min	-.003	2	-.005	2	0	15	7.693e-6	3	755.784	1	NC	1
529		18	max	.005	3	.089	1	.009	1	8.457e-3	1	NC	5	NC	1
530			min	-.003	2	-.048	3	0	15	-2.731e-3	3	1589.818	1	NC	1
531		19	max	.005	3	.173	1	0	15	1.651e-2	2	NC	1	NC	1
532			min	-.003	2	-.096	3	-.001	1	-5.563e-3	3	NC	1	NC	1
533	M5	1	max	.023	3	.363	1	0	1	0	1	NC	1	NC	1
534			min	-.016	2	-.011	3	0	1	0	1	NC	1	NC	1
535		2	max	.023	3	.18	1	0	1	0	1	NC	5	NC	1
536			min	-.016	2	-.004	3	0	1	0	1	735.355	1	NC	1
537		3	max	.023	3	.034	3	0	1	0	1	NC	15	NC	1
538			min	-.016	2	-.031	2	0	1	0	1	341.986	1	NC	1
539		4	max	.023	3	.129	3	0	1	0	1	6757.225	15	NC	1
540			min	-.016	2	-.289	1	0	1	0	1	206.426	1	NC	1
541		5	max	.022	3	.264	3	0	1	0	1	4711.947	15	NC	1
542			min	-.015	2	-.574	1	0	1	0	1	143.578	1	NC	1
543		6	max	.022	3	.418	3	0	1	0	1	3617.767	15	NC	1
544			min	-.015	2	-.86	1	0	1	0	1	109.999	1	NC	1
545		7	max	.021	3	.57	3	0	1	0	1	2987.512	15	NC	1
546			min	-.015	2	-1.12	1	0	1	0	1	90.676	1	NC	1
547		8	max	.021	3	.698	3	0	1	0	1	2621.891	15	NC	1
548			min	-.014	2	-1.329	1	0	1	0	1	79.475	1	NC	1
549		9	max	.021	3	.781	3	0	1	0	1	2434.485	15	NC	1
550			min	-.014	2	-1.461	1	0	1	0	1	73.74	1	NC	1
551		10	max	.02	3	.811	3	0	1	0	1	2377.983	15	NC	1
552			min	-.014	2	-1.505	1	0	1	0	1	72.033	1	NC	1
553		11	max	.02	3	.791	3	0	1	0	1	2434.572	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.014	2	-1.46	1	0	1	0	1	73.847	1	NC	1
555		12	max	.019	3	.723	3	0	1	0	1	2622.104	15	NC	1
556			min	-.013	2	-1.325	1	0	1	0	1	79.831	1	NC	1
557		13	max	.019	3	.612	3	0	1	0	1	2987.953	15	NC	1
558			min	-.013	2	-1.11	1	0	1	0	1	91.609	1	NC	1
559		14	max	.018	3	.472	3	0	1	0	1	3618.642	15	NC	1
560			min	-.013	2	-.843	1	0	1	0	1	112.115	1	NC	1
561		15	max	.018	3	.316	3	0	1	0	1	4713.693	15	NC	1
562			min	-.013	2	-.553	1	0	1	0	1	148.211	1	NC	1
563		16	max	.017	3	.158	3	0	1	0	1	6760.909	15	NC	1
564			min	-.013	2	-.267	1	0	1	0	1	216.92	1	NC	1
565		17	max	.017	3	.011	3	0	1	0	1	NC	15	NC	1
566			min	-.012	2	-.016	2	0	1	0	1	367.771	1	NC	1
567		18	max	.017	3	.183	1	0	1	0	1	NC	5	NC	1
568			min	-.012	2	-.115	3	0	1	0	1	805.317	1	NC	1
569		19	max	.017	3	.349	1	0	1	0	1	NC	1	NC	1
570			min	-.012	2	-.228	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.008	3	.178	1	0	15	1.94e-2	3	NC	1	NC	1
572			min	-.004	2	-.025	3	0	1	-1.384e-2	1	NC	1	NC	1
573		2	max	.008	3	.088	1	.009	1	9.631e-3	3	NC	5	NC	1
574			min	-.004	2	-.012	3	0	15	-6.673e-3	1	1502.184	1	NC	1
575		3	max	.008	3	.011	3	.013	1	2.83e-4	1	NC	5	NC	2
576			min	-.004	2	-.01	2	0	15	6.608e-7	10	721.125	1	9748.617	1
577		4	max	.007	3	.05	3	.012	1	3.858e-3	3	NC	15	NC	1
578			min	-.004	2	-.119	1	0	15	-4.761e-3	1	453.197	1	NC	1
579		5	max	.007	3	.102	3	.008	1	7.619e-3	3	9546.857	15	NC	1
580			min	-.004	2	-.235	1	0	15	-9.805e-3	1	325.654	1	NC	1
581		6	max	.007	3	.158	3	.004	1	1.138e-2	3	7536.806	15	NC	1
582			min	-.003	2	-.348	1	0	15	-1.485e-2	1	255.583	1	NC	1
583		7	max	.007	3	.212	3	0	3	1.514e-2	3	6350.97	15	NC	1
584			min	-.003	2	-.45	1	0	1	-1.989e-2	1	214.332	1	NC	1
585		8	max	.007	3	.257	3	0	15	1.89e-2	3	5649.728	15	NC	1
586			min	-.003	2	-.53	1	-.001	1	-2.494e-2	1	189.983	1	NC	1
587		9	max	.007	3	.287	3	0	1	1.909e-2	3	5283.179	15	NC	1
588			min	-.003	2	-.581	1	0	15	-2.74e-2	1	177.306	1	NC	1
589		10	max	.007	3	.298	3	0	15	1.689e-2	3	5171.239	15	NC	1
590			min	-.003	2	-.598	1	0	1	-2.815e-2	1	173.502	1	NC	1
591		11	max	.006	3	.291	3	0	15	1.469e-2	3	5282.975	15	NC	1
592			min	-.003	2	-.58	1	0	1	-2.889e-2	1	177.531	1	NC	1
593		12	max	.006	3	.267	3	.001	1	1.238e-2	3	5649.302	15	NC	1
594			min	-.003	2	-.529	1	0	15	-2.721e-2	1	190.674	1	NC	1
595		13	max	.006	3	.227	3	0	1	9.902e-3	3	6350.234	15	NC	1
596			min	-.003	2	-.446	1	0	15	-2.193e-2	1	216.023	1	NC	1
597		14	max	.006	3	.177	3	0	15	7.424e-3	3	7535.572	15	NC	1
598			min	-.003	2	-.343	1	-.003	1	-1.664e-2	1	259.199	1	NC	1
599		15	max	.006	3	.12	3	0	15	4.947e-3	3	9544.739	15	NC	1
600			min	-.003	2	-.229	1	-.008	1	-1.136e-2	1	333.07	1	NC	1
601		16	max	.006	3	.061	3	0	15	2.47e-3	3	NC	15	NC	1
602			min	-.003	2	-.113	1	-.011	1	-6.073e-3	1	468.702	1	NC	1
603		17	max	.005	3	.004	3	0	15	-7.693e-6	3	NC	5	NC	1
604			min	-.003	2	-.005	2	-.012	1	-7.879e-4	1	755.784	1	NC	1
605		18	max	.005	3	.089	1	0	15	2.731e-3	3	NC	5	NC	1
606			min	-.003	2	-.048	3	-.009	1	-8.457e-3	1	1589.818	1	NC	1
607		19	max	.005	3	.173	1	.001	1	5.563e-3	3	NC	1	NC	1
608			min	-.003	2	-.096	3	0	15	-1.651e-2	2	NC	1	NC	1



Anchor Designer™  
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Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

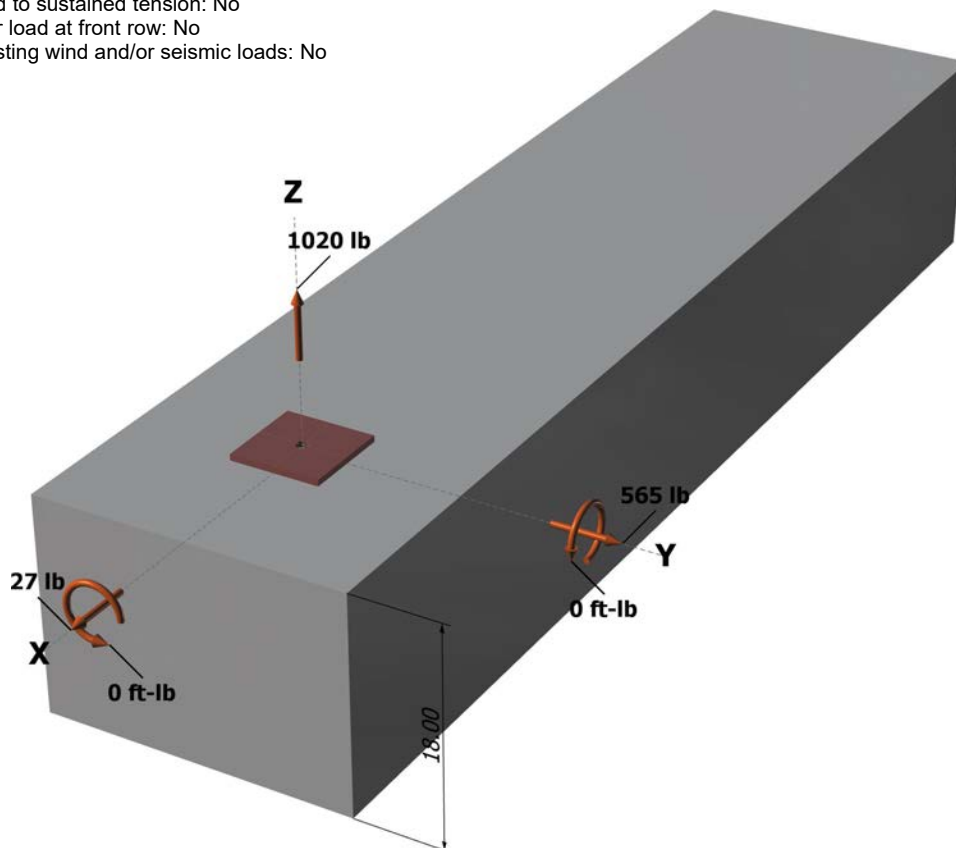
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

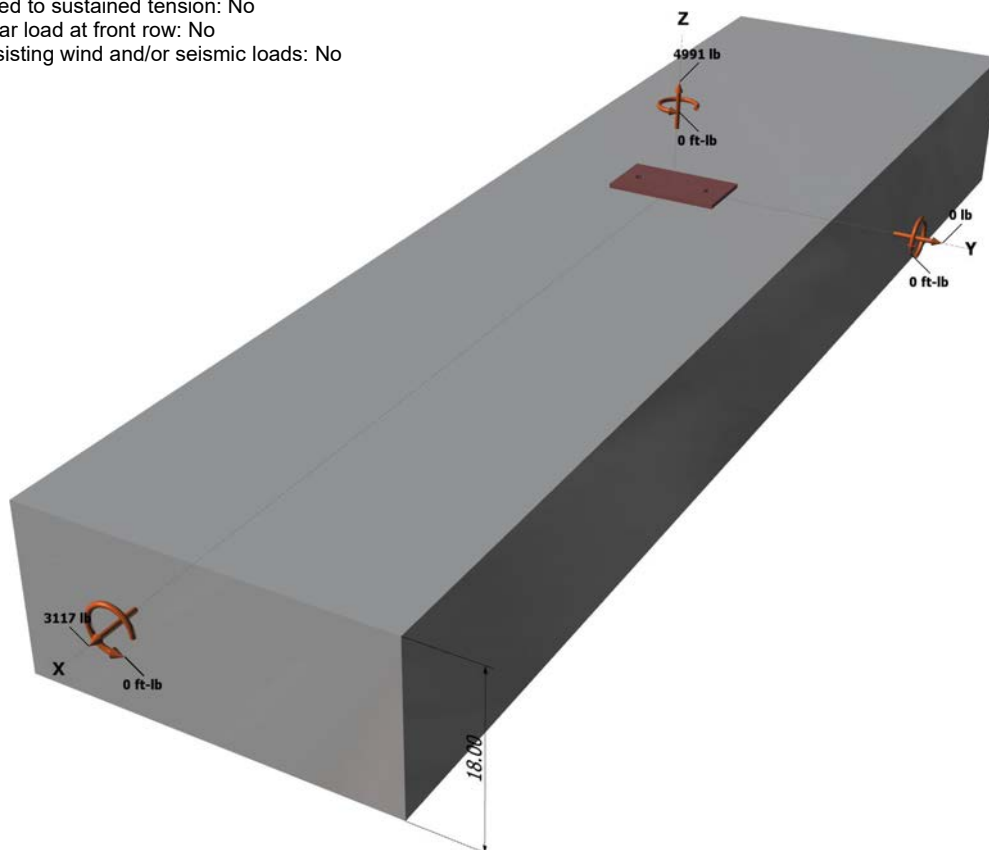
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

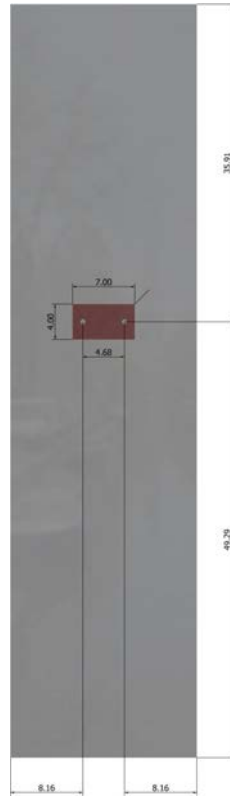
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



**Anchor Designer™**  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

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Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.