



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

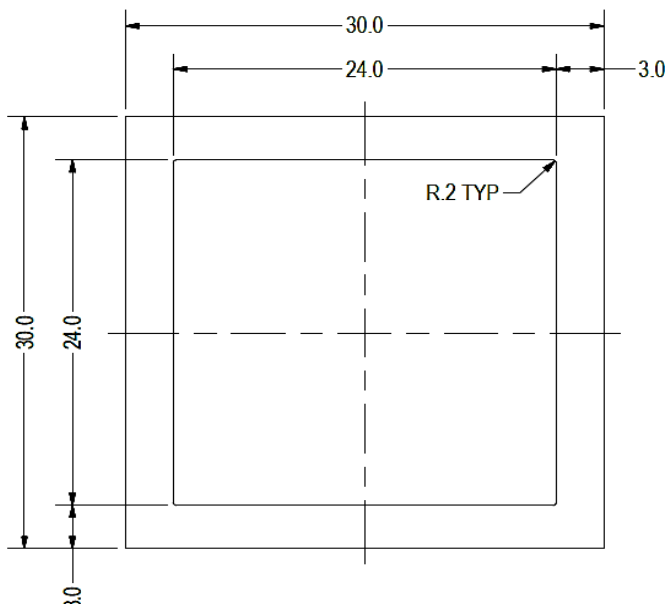
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

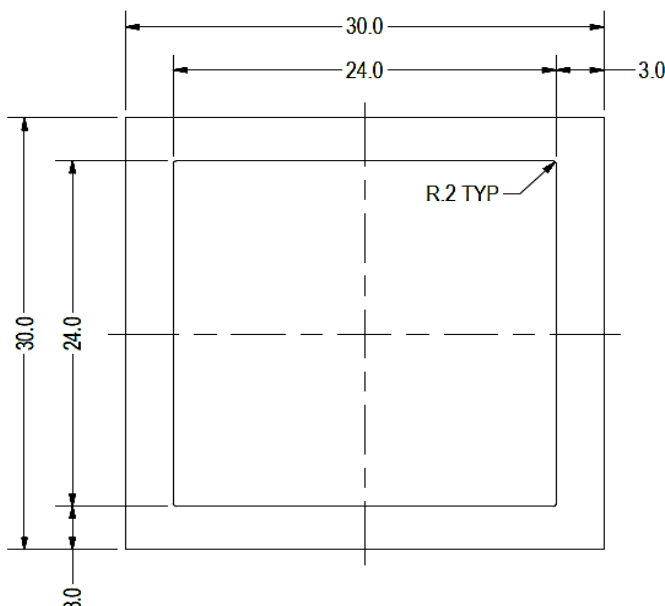
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	0.994 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>8%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.773 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>20%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.852 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<b>19%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.005 k-ft
$P_n$ =	0.060 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>11%</b>



A cross brace kit is required every 17 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

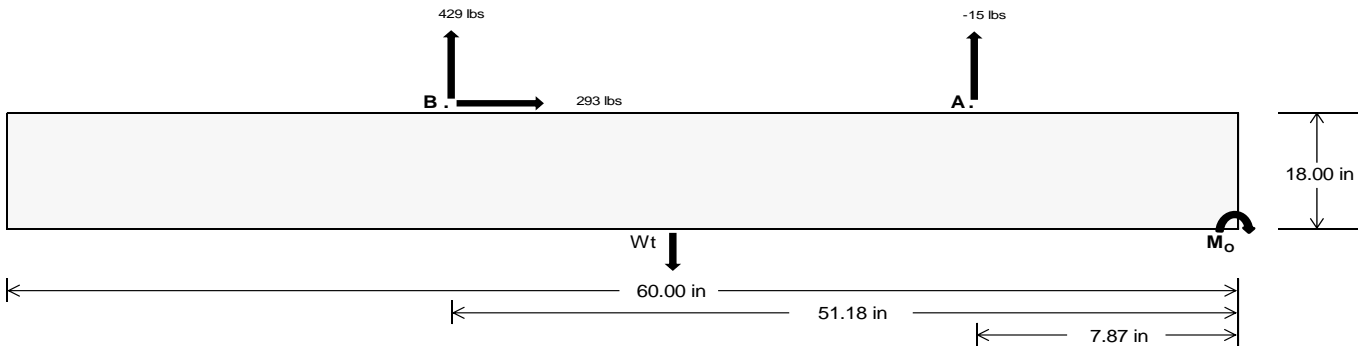
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>33.82</u>	<u>1864.99</u>	k
Compressive Load =	<u>1291.98</u>	<u>1371.60</u>	k
Lateral Load =	<u>4.85</u>	<u>1270.64</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 27117.2$  in-lbs  
Resisting Force Required = 903.91 lbs  
S.F. = 1.67  
Weight Required = 1506.51 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 292.97 lbs  
Friction = 0.4  
Weight Required = 732.44 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 292.97 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

	Ballast Width			
	21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	514 lbs	514 lbs	514 lbs	514 lbs	358 lbs	358 lbs	358 lbs	358 lbs	603 lbs	603 lbs	603 lbs	603 lbs	31 lbs	31 lbs	31 lbs	31 lbs
$F_B$	348 lbs	348 lbs	348 lbs	348 lbs	570 lbs	570 lbs	570 lbs	570 lbs	652 lbs	652 lbs	652 lbs	652 lbs	-858 lbs	-858 lbs	-858 lbs	-858 lbs
$F_V$	68 lbs	68 lbs	68 lbs	68 lbs	538 lbs	538 lbs	538 lbs	538 lbs	449 lbs	449 lbs	449 lbs	449 lbs	-586 lbs	-586 lbs	-586 lbs	-586 lbs
$P_{total}$	2765 lbs	2856 lbs	2946 lbs	3037 lbs	2831 lbs	2922 lbs	3013 lbs	3103 lbs	3158 lbs	3249 lbs	3340 lbs	3430 lbs	314 lbs	369 lbs	423 lbs	477 lbs
$M$	437 lbs-ft	437 lbs-ft	437 lbs-ft	437 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	693 lbs-ft	693 lbs-ft	693 lbs-ft	693 lbs-ft
$e$	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.20 ft	1.88 ft	1.64 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	256.1 psf	254.3 psf	252.7 psf	251.3 psf	260.3 psf	258.3 psf	256.6 psf	254.9 psf	274.2 psf	271.6 psf	269.2 psf	267.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	375.9 psf	368.7 psf	362.2 psf	356.1 psf	386.9 psf	379.2 psf	372.1 psf	365.7 psf	447.8 psf	437.3 psf	427.7 psf	419.0 psf	404.5 psf	215.8 psf	170.6 psf	151.7 psf

Maximum Bearing Pressure = 448 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

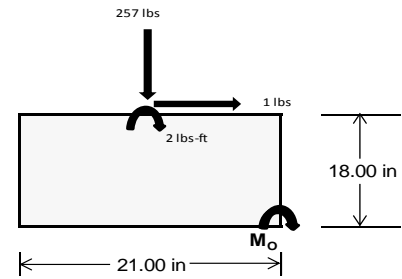
## Overturning Check

$M_o = 221.5 \text{ ft-lbs}$   
 Resisting Force Required = 253.14 lbs  
 S.F. = 1.67  
 Weight Required = 421.90 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	86 lbs	216 lbs	82 lbs	262 lbs	721 lbs	257 lbs	25 lbs	63 lbs	24 lbs
$F_v$	5 lbs	5 lbs	0 lbs	19 lbs	18 lbs	1 lbs	2 lbs	1 lbs	0 lbs
$P_{total}$	2442 lbs	2572 lbs	2438 lbs	2505 lbs	2964 lbs	2500 lbs	714 lbs	752 lbs	713 lbs
$M$	9 lbs-ft	7 lbs-ft	0 lbs-ft	33 lbs-ft	27 lbs-ft	4 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.74 ft	1.75 ft	1.72 ft	1.73 ft	1.75 ft	1.74 ft	1.74 ft	1.75 ft
$f_{min}$	275.7 sqft	291.1 sqft	278.4 sqft	273.4 sqft	328.1 sqft	284.3 sqft	80.7 sqft	85.1 sqft	81.4 sqft
$f_{max}$	282.6 psf	296.8 psf	278.8 psf	299.2 psf	349.4 psf	287.2 psf	82.5 psf	86.8 psf	81.5 psf



Maximum Bearing Pressure = 349 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

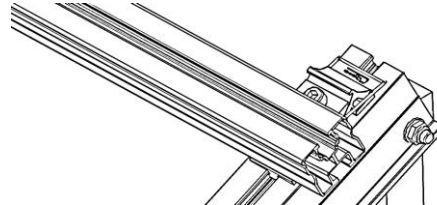
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.423 k
Allowable Uplift =	1.214 k
Utilization =	<u>35%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.087 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.994 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

#### Diagonal Strut

Maximum Axial Load =	0.773 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



#### Rear Strut

Maximum Axial Load =	1.170 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

#### Bracing

Maximum Axial Load =	0.060 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	33.11 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.662 in
Max Drift, $\Delta_{MAX}$ =	0.064 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$226.543$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$235.251$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.211 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

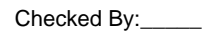
Checked By: \_\_\_\_\_

### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	246.143	2	307.809	2	-.004	15	0	15	0	1	0	1
2		min	-304.485	3	-436.154	3	-.165	1	0	1	0	1	0	1
3	N7	max	.026	3	400.51	1	-.09	15	0	15	0	1	0	1
4		min	-.171	2	16.818	15	-1.746	1	-.003	1	0	1	0	1
5	N15	max	.211	3	993.834	1	.676	1	.001	1	0	1	0	1
6		min	-1.698	2	37.073	15	-.462	3	0	3	0	1	0	1
7	N16	max	919.273	2	1055.078	2	-.21	10	0	1	0	1	0	1
8		min	-977.418	3	-1434.606	3	-52.708	3	0	3	0	1	0	1
9	N23	max	.027	3	400.17	1	3.734	1	.006	1	0	1	0	1
10		min	-.171	2	16.976	15	.182	15	0	15	0	1	0	1
11	N24	max	246.647	2	312.143	2	53.056	3	.002	1	0	1	0	1
12		min	-304.636	3	-433.707	3	.021	10	0	3	0	1	0	1
13	Totals:	max	1410.024	2	3246.341	1	0	3						
14		min	-1586.274	3	-2088.697	3	0	1						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	261.538	1	.677	4	.499	1	0	15	0	12	0	1
2			min	-360.084	3	.159	15	-.029	3	-.001	1	0	1	0	1
3		2	max	261.673	1	.62	4	.499	1	0	15	0	15	0	15
4			min	-359.983	3	.146	15	-.029	3	-.001	1	0	1	0	4
5		3	max	261.808	1	.562	4	.499	1	0	15	0	15	0	15
6			min	-359.881	3	.132	15	-.029	3	-.001	1	0	1	0	4
7		4	max	261.943	1	.505	4	.499	1	0	15	0	15	0	15
8			min	-359.78	3	.119	15	-.029	3	-.001	1	0	1	0	4
9		5	max	262.077	1	.447	4	.499	1	0	15	0	1	0	15
10			min	-359.679	3	.105	15	-.029	3	-.001	1	0	3	0	4
11		6	max	262.212	1	.39	4	.499	1	0	15	0	1	0	15
12			min	-359.578	3	.092	15	-.029	3	-.001	1	0	3	0	4
13		7	max	262.347	1	.332	4	.499	1	0	15	0	1	0	15
14			min	-359.477	3	.078	15	-.029	3	-.001	1	0	3	0	4
15		8	max	262.482	1	.275	4	.499	1	0	15	0	1	0	15
16			min	-359.376	3	.065	15	-.029	3	-.001	1	0	3	0	4
17		9	max	262.617	1	.217	4	.499	1	0	15	0	1	0	15
18			min	-359.275	3	.051	15	-.029	3	-.001	1	0	3	0	4
19		10	max	262.752	1	.16	4	.499	1	0	15	0	1	0	15
20			min	-359.173	3	.038	15	-.029	3	-.001	1	0	3	0	4
21		11	max	262.887	1	.108	2	.499	1	0	15	0	1	0	15
22			min	-359.072	3	.017	12	-.029	3	-.001	1	0	3	0	4
23		12	max	263.022	1	.063	2	.499	1	0	15	0	1	0	15
24			min	-358.971	3	-.013	3	-.029	3	-.001	1	0	3	0	4
25		13	max	263.156	1	.018	2	.499	1	0	15	0	1	0	15
26			min	-358.87	3	-.046	3	-.029	3	-.001	1	0	3	0	4
27		14	max	263.291	1	-.016	15	.499	1	0	15	0	1	0	15
28			min	-358.769	3	-.08	3	-.029	3	-.001	1	0	3	0	4
29		15	max	263.426	1	-.03	15	.499	1	0	15	.001	1	0	15
30			min	-358.668	3	-.128	4	-.029	3	-.001	1	0	3	0	4
31		16	max	263.561	1	-.043	15	.499	1	0	15	.001	1	0	15
32			min	-358.567	3	-.185	4	-.029	3	-.001	1	0	3	0	4
33		17	max	263.696	1	-.057	15	.499	1	0	15	.001	1	0	15
34			min	-358.465	3	-.242	4	-.029	3	-.001	1	0	3	0	4
35		18	max	263.831	1	-.07	15	.499	1	0	15	.001	1	0	15
36			min	-358.364	3	-.3	4	-.029	3	-.001	1	0	3	0	4
37		19	max	263.966	1	-.084	15	.499	1	0	15	.001	1	0	15









Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1167.721	3	-.353	4	-.146	3	0	15	0	3	0	2
153	M7	1	max	772.583	2	1.74	4	.027	3	0	2	0	2	2
154		min	-672.763	3	.409	15	-.005	2	0	3	0	3	0	3
155		2	max	772.513	2	1.564	4	.027	3	0	2	0	2	2
156		min	-672.815	3	.367	15	-.005	2	0	3	0	3	0	3
157		3	max	772.443	2	1.388	4	.027	3	0	2	0	2	2
158		min	-672.868	3	.326	15	-.005	2	0	3	0	3	0	3
159		4	max	772.373	2	1.211	4	.027	3	0	2	0	2	2
160		min	-672.92	3	.284	15	-.005	2	0	3	0	3	0	3
161		5	max	772.303	2	1.035	4	.027	3	0	2	0	2	15
162		min	-672.973	3	.243	15	-.005	2	0	3	0	3	0	3
163		6	max	772.233	2	.859	4	.027	3	0	2	0	2	15
164		min	-673.025	3	.202	15	-.005	2	0	3	0	3	0	3
165		7	max	772.163	2	.682	4	.027	3	0	2	0	2	15
166		min	-673.078	3	.16	15	-.005	2	0	3	0	3	0	4
167		8	max	772.093	2	.506	4	.027	3	0	2	0	2	15
168		min	-673.13	3	.119	15	-.005	2	0	3	0	3	-.001	4
169		9	max	772.023	2	.349	2	.027	3	0	2	0	2	15
170		min	-673.183	3	.054	12	-.005	2	0	3	0	3	-.001	4
171		10	max	771.953	2	.211	2	.027	3	0	2	0	2	15
172		min	-673.235	3	-.033	3	-.005	2	0	3	0	3	-.001	4
173		11	max	771.883	2	.074	2	.027	3	0	2	0	2	15
174		min	-673.288	3	-.137	3	-.005	2	0	3	0	3	-.001	4
175		12	max	771.813	2	-.047	15	.027	3	0	2	0	2	15
176		min	-673.34	3	-.24	3	-.005	2	0	3	0	3	-.001	4
177		13	max	771.743	2	-.089	15	.027	3	0	2	0	2	15
178		min	-673.393	3	-.376	4	-.005	2	0	3	0	3	-.001	4
179		14	max	771.673	2	-.13	15	.027	3	0	2	0	2	15
180		min	-673.445	3	-.552	4	-.005	2	0	3	0	3	-.001	4
181		15	max	771.603	2	-.172	15	.027	3	0	2	0	2	15
182		min	-673.498	3	-.729	4	-.005	2	0	3	0	3	0	4
183		16	max	771.533	2	-.213	15	.027	3	0	2	0	2	15
184		min	-673.55	3	-.905	4	-.005	2	0	3	0	3	0	4
185		17	max	771.463	2	-.254	15	.027	3	0	2	0	2	15
186		min	-673.603	3	-1.082	4	-.005	2	0	3	0	3	0	4
187		18	max	771.393	2	-.296	15	.027	3	0	2	0	2	15
188		min	-673.655	3	-1.258	4	-.005	2	0	3	0	3	0	4
189		19	max	771.323	2	-.337	15	.027	3	0	2	0	2	1
190		min	-673.708	3	-1.434	4	-.005	2	0	3	0	3	0	1
191	M8	1	max	992.669	1	0	1	.807	1	0	1	0	15	0
192		min	36.722	15	0	1	-.471	3	0	1	0	1	0	1
193		2	max	992.734	1	0	1	.807	1	0	1	0	1	0
194		min	36.741	15	0	1	-.471	3	0	1	0	3	0	1
195		3	max	992.799	1	0	1	.807	1	0	1	0	1	0
196		min	36.761	15	0	1	-.471	3	0	1	0	3	0	1
197		4	max	992.863	1	0	1	.807	1	0	1	0	1	0
198		min	36.78	15	0	1	-.471	3	0	1	0	3	0	1
199		5	max	992.928	1	0	1	.807	1	0	1	0	1	0
200		min	36.8	15	0	1	-.471	3	0	1	0	3	0	1
201		6	max	992.993	1	0	1	.807	1	0	1	0	1	0
202		min	36.819	15	0	1	-.471	3	0	1	0	3	0	1
203		7	max	993.058	1	0	1	.807	1	0	1	0	1	0
204		min	36.839	15	0	1	-.471	3	0	1	0	3	0	1
205		8	max	993.122	1	0	1	.807	1	0	1	0	1	0
206		min	36.858	15	0	1	-.471	3	0	1	0	3	0	1
207		9	max	993.187	1	0	1	.807	1	0	1	0	1	0
208		min	36.878	15	0	1	-.471	3	0	1	0	3	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	993.252	1	0	1	.807	1	0	1	0	1	0	1
210		min	36.897	15	0	1	-.471	3	0	1	0	3	0	1
211	11	max	993.316	1	0	1	.807	1	0	1	0	1	0	1
212		min	36.917	15	0	1	-.471	3	0	1	0	3	0	1
213	12	max	993.381	1	0	1	.807	1	0	1	0	1	0	1
214		min	36.936	15	0	1	-.471	3	0	1	0	3	0	1
215	13	max	993.446	1	0	1	.807	1	0	1	0	1	0	1
216		min	36.956	15	0	1	-.471	3	0	1	0	3	0	1
217	14	max	993.511	1	0	1	.807	1	0	1	0	1	0	1
218		min	36.975	15	0	1	-.471	3	0	1	0	3	0	1
219	15	max	993.575	1	0	1	.807	1	0	1	.001	1	0	1
220		min	36.995	15	0	1	-.471	3	0	1	0	3	0	1
221	16	max	993.64	1	0	1	.807	1	0	1	.001	1	0	1
222		min	37.015	15	0	1	-.471	3	0	1	0	3	0	1
223	17	max	993.705	1	0	1	.807	1	0	1	.001	1	0	1
224		min	37.034	15	0	1	-.471	3	0	1	0	3	0	1
225	18	max	993.769	1	0	1	.807	1	0	1	.001	1	0	1
226		min	37.054	15	0	1	-.471	3	0	1	0	3	0	1
227	19	max	993.834	1	0	1	.807	1	0	1	.001	1	0	1
228		min	37.073	15	0	1	-.471	3	0	1	0	3	0	1
229	M10	1	max	274.98	1	.673	.005	3	.001	1	0	1	0	1
230		min	-333.007	3	.159	15	-.214	1	0	3	0	3	0	1
231	2	max	275.115	1	.615	4	.005	3	.001	1	0	1	0	15
232		min	-332.906	3	.145	15	-.214	1	0	3	0	3	0	4
233	3	max	275.25	1	.558	4	.005	3	.001	1	0	1	0	15
234		min	-332.805	3	.132	15	-.214	1	0	3	0	3	0	4
235	4	max	275.385	1	.5	4	.005	3	.001	1	0	1	0	15
236		min	-332.704	3	.118	15	-.214	1	0	3	0	3	0	4
237	5	max	275.519	1	.443	4	.005	3	.001	1	0	1	0	15
238		min	-332.603	3	.105	15	-.214	1	0	3	0	3	0	4
239	6	max	275.654	1	.385	4	.005	3	.001	1	0	1	0	15
240		min	-332.501	3	.091	15	-.214	1	0	3	0	3	0	4
241	7	max	275.789	1	.328	4	.005	3	.001	1	0	1	0	15
242		min	-332.4	3	.078	15	-.214	1	0	3	0	3	0	4
243	8	max	275.924	1	.27	4	.005	3	.001	1	0	1	0	15
244		min	-332.299	3	.064	15	-.214	1	0	3	0	3	0	4
245	9	max	276.059	1	.213	4	.005	3	.001	1	0	1	0	15
246		min	-332.198	3	.051	15	-.214	1	0	3	0	3	0	4
247	10	max	276.194	1	.155	4	.005	3	.001	1	0	1	0	15
248		min	-332.097	3	.037	15	-.214	1	0	3	0	3	0	4
249	11	max	276.329	1	.108	2	.005	3	.001	1	0	11	0	15
250		min	-331.996	3	.024	15	-.214	1	0	3	0	3	0	4
251	12	max	276.463	1	.063	2	.005	3	.001	1	0	15	0	15
252		min	-331.895	3	.008	12	-.214	1	0	3	0	3	0	4
253	13	max	276.598	1	.018	2	.005	3	.001	1	0	15	0	15
254		min	-331.793	3	-.023	3	-.214	1	0	3	0	3	0	4
255	14	max	276.733	1	-.017	15	.005	3	.001	1	0	15	0	15
256		min	-331.692	3	-.074	4	-.214	1	0	3	0	3	0	4
257	15	max	276.868	1	-.031	15	.005	3	.001	1	0	15	0	15
258		min	-331.591	3	-.132	4	-.214	1	0	3	0	1	0	4
259	16	max	277.003	1	-.044	15	.005	3	.001	1	0	15	0	15
260		min	-331.49	3	-.189	4	-.214	1	0	3	0	1	0	4
261	17	max	277.138	1	-.058	15	.005	3	.001	1	0	15	0	15
262		min	-331.389	3	-.247	4	-.214	1	0	3	0	1	0	4
263	18	max	277.273	1	-.071	15	.005	3	.001	1	0	15	0	15
264		min	-331.288	3	-.304	4	-.214	1	0	3	0	1	0	4
265	19	max	277.408	1	-.085	15	.005	3	.001	1	0	15	0	15







Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-150.128	1	-163.179	3	-81.994	1	0	2	-.156	1	0	3
381	M5	max	329.335	1	1112.179	3	-.077	10	0	1	.005	3	0	3
382		min	12.641	12	-851.007	1	-47.347	3	0	3	0	10	0	2
383		max	329.495	1	1112.007	3	-.077	10	0	1	0	2	.184	1
384		min	12.721	12	-851.236	1	-47.347	3	0	3	-.005	3	-.241	3
385		max	361.278	3	5.731	9	5.466	3	0	3	0	2	.365	1
386		min	-70.493	2	-103.508	2	-.407	2	0	1	-.015	3	-.477	3
387		max	361.399	3	5.541	9	5.466	3	0	3	0	2	.375	1
388		min	-70.333	2	-103.736	2	-.407	2	0	1	-.014	3	-.469	3
389		max	361.519	3	5.35	9	5.466	3	0	3	0	2	.392	2
390		min	-70.172	2	-103.965	2	-.407	2	0	1	-.013	3	-.462	3
391		max	361.639	3	5.159	9	5.466	3	0	3	0	2	.415	2
392		min	-70.012	2	-104.194	2	-.407	2	0	1	-.011	3	-.454	3
393		max	361.759	3	4.969	9	5.466	3	0	3	0	2	.438	2
394		min	-69.852	2	-104.422	2	-.407	2	0	1	-.01	3	-.446	3
395		max	361.879	3	4.778	9	5.466	3	0	3	0	2	.46	2
396		min	-69.692	2	-104.651	2	-.407	2	0	1	-.009	3	-.439	3
397		max	361.999	3	4.588	9	5.466	3	0	3	0	2	.483	2
398		min	-69.532	2	-104.88	2	-.407	2	0	1	-.008	3	-.431	3
399		max	362.119	3	4.397	9	5.466	3	0	3	0	10	.506	2
400		min	-69.372	2	-105.109	2	-.407	2	0	1	-.007	3	-.423	3
401		max	362.239	3	4.206	9	5.466	3	0	3	0	10	.529	2
402		min	-69.211	2	-105.337	2	-.407	2	0	1	-.006	3	-.415	3
403		max	362.36	3	4.016	9	5.466	3	0	3	0	10	.552	2
404		min	-69.051	2	-105.566	2	-.407	2	0	1	-.004	3	-.407	3
405		max	362.48	3	3.825	9	5.466	3	0	3	0	10	.574	2
406		min	-68.891	2	-105.795	2	-.407	2	0	1	-.003	3	-.399	3
407		max	362.6	3	3.635	9	5.466	3	0	3	0	10	.597	2
408		min	-68.731	2	-106.024	2	-.407	2	0	1	-.002	3	-.391	3
409		max	362.72	3	3.444	9	5.466	3	0	3	0	15	.62	2
410		min	-68.571	2	-106.252	2	-.407	2	0	1	-.002	1	-.383	3
411		max	294.224	2	578.051	2	5.45	3	0	1	0	3	.638	2
412		min	6.067	15	-633.084	3	-.444	2	0	15	-.001	1	-.37	3
413		max	294.384	2	577.823	2	5.45	3	0	1	.001	3	.512	2
414		min	6.115	15	-633.256	3	-.444	2	0	15	-.001	1	-.233	3
415		max	-13.654	12	1187.431	2	4.971	3	0	15	.002	3	.257	2
416		min	-330.111	1	-535.972	3	-.103	2	0	1	0	1	-.116	3
417		max	-13.574	12	1187.203	2	4.971	3	0	15	.003	3	0	3
418		min	-329.951	1	-536.143	3	-.103	2	0	1	0	2	0	2
419	M9	max	149.915	1	336.368	3	99.395	1	0	3	-.007	15	0	2
420		min	6.792	15	-257.031	1	4.888	15	0	1	-.156	1	0	3
421		max	150.076	1	336.197	3	99.395	1	0	3	-.004	12	.056	1
422		min	6.84	15	-257.26	1	4.888	15	0	1	-.134	1	-.073	3
423		max	115.267	3	6.927	9	75.671	1	0	1	.006	3	.111	1
424		min	-13.008	10	-27.683	2	1.376	12	0	12	-.112	1	-.145	3
425		max	115.387	3	6.736	9	75.671	1	0	1	.006	3	.112	1
426		min	-12.875	10	-27.912	2	1.376	12	0	12	-.095	1	-.143	3
427		max	115.507	3	6.545	9	75.671	1	0	1	.006	3	.118	2
428		min	-12.741	10	-28.141	2	1.376	12	0	12	-.079	1	-.141	3
429		max	115.627	3	6.355	9	75.671	1	0	1	.007	3	.124	2
430		min	-12.608	10	-28.369	2	1.376	12	0	12	-.062	1	-.139	3
431		max	115.747	3	6.164	9	75.671	1	0	1	.007	3	.13	2
432		min	-12.474	10	-28.598	2	1.376	12	0	12	-.046	1	-.137	3
433		max	115.867	3	5.973	9	75.671	1	0	1	.007	3	.136	2
434		min	-12.341	10	-28.827	2	1.376	12	0	12	-.03	1	-.135	3
435		max	115.988	3	5.783	9	75.671	1	0	1	.008	3	.142	2
436		min	-12.207	10	-29.056	2	1.376	12	0	12	-.013	1	-.132	3







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	3.647	15	-256.608	1	6.825	15	0	2	.007	15	0	3
495	M16	1	max	-1.73	12	360.337	2	-6.783	15	0	3	.154	1	0	2
496			min	-79.998	1	-163.208	3	-149.779	1	0	2	.007	15	0	3
497		2	max	-1.73	12	254.222	2	-5.203	15	0	3	.047	1	.112	3
498			min	-79.998	1	-115.276	3	-114.788	1	0	2	.002	15	-.247	2
499		3	max	-1.73	12	148.107	2	-3.624	15	0	3	-.001	12	.186	3
500			min	-79.998	1	-67.344	3	-79.797	1	0	2	-.031	1	-.41	2
501		4	max	-1.73	12	41.992	2	-2.044	15	0	3	-.004	15	.221	3
502			min	-79.998	1	-19.412	3	-44.806	1	0	2	-.081	1	-.486	2
503		5	max	-1.73	12	28.52	3	-.465	15	0	3	-.005	15	.217	3
504			min	-79.998	1	-64.123	2	-9.814	1	0	2	-.103	1	-.477	2
505		6	max	-1.73	12	76.452	3	25.177	1	0	3	-.004	15	.175	3
506			min	-79.998	1	-170.238	2	.532	12	0	2	-.097	1	-.383	2
507		7	max	-1.73	12	124.384	3	60.168	1	0	3	-.003	15	.094	3
508			min	-79.998	1	-276.353	2	2.065	12	0	2	-.063	1	-.203	2
509		8	max	-1.73	12	172.316	3	95.159	1	0	3	.002	2	.062	2
510			min	-79.998	1	-382.468	2	3.597	12	0	2	-.003	3	-.026	3
511		9	max	-1.73	12	220.248	3	130.15	1	0	3	.091	1	.413	2
512			min	-79.998	1	-488.583	2	5.13	12	0	2	.002	12	-.184	3
513		10	max	-3.741	15	-12.647	15	165.142	1	0	15	.21	1	.85	2
514			min	-81.716	1	-594.698	2	-10.323	3	0	2	.008	12	-.381	3
515		11	max	-3.741	15	488.583	2	-5.381	12	0	2	.091	1	.413	2
516			min	-81.716	1	-220.248	3	-129.785	1	0	3	.003	12	-.184	3
517		12	max	-3.741	15	382.468	2	-3.848	12	0	2	.002	2	.062	2
518			min	-81.716	1	-172.316	3	-94.794	1	0	3	0	3	-.026	3
519		13	max	-3.741	15	276.353	2	-2.316	12	0	2	-.003	12	.094	3
520			min	-81.716	1	-124.384	3	-59.803	1	0	3	-.062	1	-.203	2
521		14	max	-3.741	15	170.238	2	-.783	12	0	2	-.004	12	.175	3
522			min	-81.716	1	-76.452	3	-24.812	1	0	3	-.096	1	-.383	2
523		15	max	-3.741	15	64.123	2	10.18	1	0	2	-.004	12	.217	3
524			min	-81.716	1	-28.52	3	.479	15	0	3	-.102	1	-.477	2
525		16	max	-3.741	15	19.412	3	45.171	1	0	2	-.003	12	.221	3
526			min	-81.716	1	-41.992	2	2.058	15	0	3	-.08	1	-.486	2
527		17	max	-3.741	15	67.344	3	80.162	1	0	2	0	12	.186	3
528			min	-81.716	1	-148.107	2	3.638	15	0	3	-.029	1	-.41	2
529		18	max	-3.741	15	115.276	3	115.153	1	0	2	.049	1	.112	3
530			min	-81.716	1	-254.222	2	5.218	15	0	3	.002	15	-.247	2
531		19	max	-3.741	15	163.208	3	150.144	1	0	2	.156	1	0	2
532			min	-81.716	1	-360.337	2	6.797	15	0	3	.007	15	0	3
533	M15	1	max	0	2	2.542	4	.042	3	0	1	0	1	0	1
534			min	-58.341	3	0	2	-.034	1	0	3	0	3	0	1
535		2	max	0	2	2.259	4	.042	3	0	1	0	1	0	2
536			min	-58.417	3	0	2	-.034	1	0	3	0	3	-.001	4
537		3	max	0	2	1.977	4	.042	3	0	1	0	1	0	2
538			min	-58.492	3	0	2	-.034	1	0	3	0	3	-.002	4
539		4	max	0	2	1.694	4	.042	3	0	1	0	1	0	2
540			min	-58.568	3	0	2	-.034	1	0	3	0	3	-.003	4
541		5	max	0	2	1.412	4	.042	3	0	1	0	1	0	2
542			min	-58.643	3	0	2	-.034	1	0	3	0	3	-.004	4
543		6	max	0	2	1.13	4	.042	3	0	1	0	1	0	2
544			min	-58.719	3	0	2	-.034	1	0	3	0	3	-.004	4
545		7	max	0	2	.847	4	.042	3	0	1	0	3	0	2
546			min	-58.794	3	0	2	-.034	1	0	3	0	1	-.005	4
547		8	max	0	2	.565	4	.042	3	0	1	0	3	0	2
548			min	-58.87	3	0	2	-.034	1	0	3	0	1	-.005	4
549		9	max	0	2	.282	4	.042	3	0	1	0	3	0	2
550			min	-58.946	3	0	2	-.034	1	0	3	0	1	-.005	4



Company : Schletter, Inc.  
Designer : HCV  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	2	0	1	.042	3	0	1	0	3	0	2
552			min	-59.021	3	0	1	-.034	1	0	3	0	1	-.005	4
553		11	max	0	2	0	2	.042	3	0	1	0	3	0	2
554			min	-59.097	3	-.282	4	-.034	1	0	3	0	1	-.005	4
555		12	max	0	2	0	2	.042	3	0	1	0	3	0	2
556			min	-59.172	3	-.565	4	-.034	1	0	3	0	1	-.005	4
557		13	max	0	2	0	2	.042	3	0	1	0	3	0	2
558			min	-59.248	3	-.847	4	-.034	1	0	3	0	1	-.005	4
559		14	max	0	2	0	2	.042	3	0	1	0	3	0	2
560			min	-59.323	3	-1.13	4	-.034	1	0	3	0	1	-.004	4
561		15	max	0	2	0	2	.042	3	0	1	0	3	0	2
562			min	-59.399	3	-1.412	4	-.034	1	0	3	0	1	-.004	4
563		16	max	0	2	0	2	.042	3	0	1	0	3	0	2
564			min	-59.474	3	-1.694	4	-.034	1	0	3	0	1	-.003	4
565		17	max	0	2	0	2	.042	3	0	1	0	3	0	2
566			min	-59.55	3	-1.977	4	-.034	1	0	3	0	1	-.002	4
567		18	max	0	2	0	2	.042	3	0	1	0	3	0	2
568			min	-59.625	3	-2.259	4	-.034	1	0	3	0	1	-.001	4
569		19	max	0	2	0	2	.042	3	0	1	0	3	0	1
570			min	-59.701	3	-2.542	4	-.034	1	0	3	0	1	0	1
571	M16A	1	max	-.985	10	2.542	4	.021	1	0	3	0	3	0	1
572			min	-59.025	3	.597	15	-.017	3	0	2	0	1	0	1
573		2	max	-.901	10	2.259	4	.021	1	0	3	0	3	0	15
574			min	-58.949	3	.531	15	-.017	3	0	2	0	1	-.001	4
575		3	max	-.817	10	1.977	4	.021	1	0	3	0	3	0	15
576			min	-58.874	3	.465	15	-.017	3	0	2	0	1	-.002	4
577		4	max	-.733	10	1.694	4	.021	1	0	3	0	3	0	15
578			min	-58.798	3	.398	15	-.017	3	0	2	0	1	-.003	4
579		5	max	-.649	10	1.412	4	.021	1	0	3	0	3	0	15
580			min	-58.723	3	.332	15	-.017	3	0	2	0	1	-.004	4
581		6	max	-.565	10	1.13	4	.021	1	0	3	0	3	0	15
582			min	-58.647	3	.266	15	-.017	3	0	2	0	1	-.004	4
583		7	max	-.481	10	.847	4	.021	1	0	3	0	3	-.001	15
584			min	-58.572	3	.199	15	-.017	3	0	2	0	1	-.005	4
585		8	max	-.397	10	.565	4	.021	1	0	3	0	3	-.001	15
586			min	-58.496	3	.133	15	-.017	3	0	2	0	1	-.005	4
587		9	max	-.313	10	.282	4	.021	1	0	3	0	3	-.001	15
588			min	-58.421	3	.066	15	-.017	3	0	2	0	1	-.005	4
589		10	max	-.229	10	0	1	.021	1	0	3	0	3	-.001	15
590			min	-58.345	3	0	1	-.017	3	0	2	0	1	-.005	4
591		11	max	-.145	10	-.066	15	.021	1	0	3	0	3	-.001	15
592			min	-58.27	3	-.282	4	-.017	3	0	2	0	1	-.005	4
593		12	max	-.061	10	-.133	15	.021	1	0	3	0	3	-.001	15
594			min	-58.194	3	-.565	4	-.017	3	0	2	0	1	-.005	4
595		13	max	.023	10	-.199	15	.021	1	0	3	0	2	-.001	15
596			min	-58.119	3	-.847	4	-.017	3	0	2	0	3	-.005	4
597		14	max	.106	10	-.266	15	.021	1	0	3	0	2	0	15
598			min	-58.043	3	-1.13	4	-.017	3	0	2	0	3	-.004	4
599		15	max	.19	10	-.332	15	.021	1	0	3	0	1	0	15
600			min	-57.968	3	-1.412	4	-.017	3	0	2	0	3	-.004	4
601		16	max	.274	10	-.398	15	.021	1	0	3	0	1	0	15
602			min	-57.892	3	-1.694	4	-.017	3	0	2	0	3	-.003	4
603		17	max	.358	10	-.465	15	.021	1	0	3	0	1	0	15
604			min	-57.817	3	-1.977	4	-.017	3	0	2	0	3	-.002	4
605		18	max	.442	10	-.531	15	.021	1	0	3	0	1	0	15
606			min	-57.741	3	-2.259	4	-.017	3	0	2	0	3	-.001	4
607		19	max	.526	10	-.597	15	.021	1	0	3	0	1	0	1





Company : Schletter, Inc.  
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Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-57.665	3	-2.542	4	-.017	3	0	2	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.01	2	.015	1	-6.067e-5	15	NC	3	NC	3	
2			min	-.004	3	-.011	3	-.001	3	-1.323e-3	1	4084.842	2	2787.788	1	
3			2	max	.003	1	.009	2	.014	1	-5.797e-5	15	NC	3	NC	3
4				min	-.004	3	-.01	3	0	3	-1.265e-3	1	4472.134	2	2991.38	1
5			3	max	.002	1	.009	2	.013	1	-5.527e-5	15	NC	1	NC	3
6				min	-.003	3	-.01	3	0	3	-1.206e-3	1	4935.4	2	3232.632	1
7			4	max	.002	1	.008	2	.012	1	-5.257e-5	15	NC	1	NC	3
8				min	-.003	3	-.01	3	0	3	-1.148e-3	1	5493.442	2	3520.369	1
9			5	max	.002	1	.007	2	.011	1	-4.987e-5	15	NC	1	NC	3
10				min	-.003	3	-.009	3	0	3	-1.089e-3	1	6171.452	2	3866.374	1
11		6	max	.002	1	.006	2	.01	1	-4.717e-5	15	NC	1	NC	2	
12			min	-.003	3	-.009	3	0	3	-1.031e-3	1	7003.75	2	4286.654	1	
13		7	max	.002	1	.005	2	.009	1	-4.447e-5	15	NC	1	NC	2	
14			min	-.003	3	-.008	3	0	3	-9.726e-4	1	8037.982	2	4803.395	1	
15		8	max	.002	1	.005	2	.008	1	-4.177e-5	15	NC	1	NC	2	
16			min	-.002	3	-.008	3	0	3	-9.142e-4	1	9341.778	2	5448.071	1	
17		9	max	.002	1	.004	2	.007	1	-3.907e-5	15	NC	1	NC	2	
18			min	-.002	3	-.007	3	0	3	-8.557e-4	1	NC	1	6266.573	1	
19		10	max	.001	1	.003	2	.006	1	-3.637e-5	15	NC	1	NC	2	
20			min	-.002	3	-.007	3	0	3	-7.973e-4	1	NC	1	7328.032	1	
21		11	max	.001	1	.003	2	.005	1	-3.368e-5	15	NC	1	NC	2	
22			min	-.002	3	-.006	3	0	3	-7.388e-4	1	NC	1	8740.742	1	
23		12	max	.001	1	.002	2	.004	1	-3.098e-5	15	NC	1	NC	1	
24			min	-.001	3	-.005	3	0	3	-6.804e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.003	1	-2.828e-5	15	NC	1	NC	1	
26			min	-.001	3	-.005	3	0	3	-6.219e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-2.558e-5	15	NC	1	NC	1	
28			min	-.001	3	-.004	3	0	3	-5.635e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-2.288e-5	15	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-5.05e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-2.018e-5	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-4.466e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.748e-5	15	NC	1	NC	1	
34			min	0	3	-.002	3	0	3	-3.881e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.478e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.297e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-9.651e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.712e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.299e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	4.742e-6	12	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	1.575e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	7.069e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	1.852e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	8.349e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	2.128e-4	1	NC	1	NC	1
46				min	0	2	-.003	3	-.001	1	9.628e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	2.404e-4	1	NC	1	NC	1
48				min	0	2	-.004	3	-.001	1	1.091e-5	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	2.68e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.219e-5	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	2.956e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	1	1.347e-5	15	NC	1	NC	1
53		8 max	0	3	0	2	0	3	3.233e-4	1	NC	1	NC	1
54		min	0	2	-.006	3	0	1	1.475e-5	15	NC	1	NC	1
55		9 max	.001	3	.001	2	0	3	3.509e-4	1	NC	1	NC	1
56		min	0	2	-.007	3	0	11	1.603e-5	15	NC	1	NC	1
57		10 max	.001	3	.002	2	0	1	3.785e-4	1	NC	1	NC	1
58		min	-.001	2	-.007	3	0	15	1.731e-5	15	NC	1	NC	1
59		11 max	.001	3	.002	2	.001	1	4.061e-4	1	NC	1	NC	1
60		min	-.001	2	-.008	3	0	15	1.858e-5	15	NC	1	NC	1
61		12 max	.001	3	.003	2	.002	1	4.338e-4	1	NC	1	NC	1
62		min	-.001	2	-.008	3	0	15	1.986e-5	15	NC	1	NC	1
63		13 max	.002	3	.004	2	.003	1	4.614e-4	1	NC	1	NC	1
64		min	-.001	2	-.008	3	0	15	2.114e-5	15	NC	1	NC	1
65		14 max	.002	3	.004	2	.004	1	4.89e-4	1	NC	1	NC	1
66		min	-.002	2	-.008	3	0	15	2.242e-5	15	NC	1	NC	1
67		15 max	.002	3	.005	2	.004	1	5.166e-4	1	NC	1	NC	1
68		min	-.002	2	-.009	3	0	15	2.37e-5	15	8804.49	2	NC	1
69		16 max	.002	3	.006	2	.005	1	5.442e-4	1	NC	1	NC	2
70		min	-.002	2	-.009	3	0	15	2.498e-5	15	7449.056	2	8663.05	1
71		17 max	.002	3	.007	2	.006	1	5.719e-4	1	NC	1	NC	2
72		min	-.002	2	-.009	3	0	15	2.626e-5	15	6403.576	2	7409.618	1
73		18 max	.002	3	.008	2	.007	1	5.995e-4	1	NC	1	NC	2
74		min	-.002	2	-.009	3	0	15	2.754e-5	15	5587.31	2	6474.635	1
75		19 max	.002	3	.009	2	.008	1	6.271e-4	1	NC	3	NC	2
76		min	-.002	2	-.009	3	0	15	2.882e-5	15	4943.906	2	5759.289	1
77	M4	1 max	.002	1	.012	2	0	15	-4.346e-5	12	NC	1	NC	3
78		min	0	15	-.011	3	-.006	1	-1.009e-3	1	NC	1	3200.497	1
79		2 max	.002	1	.012	2	0	15	-4.346e-5	12	NC	1	NC	3
80		min	0	15	-.01	3	-.006	1	-1.009e-3	1	NC	1	3491.512	1
81		3 max	.002	1	.011	2	0	15	-4.346e-5	12	NC	1	NC	3
82		min	0	15	-.01	3	-.005	1	-1.009e-3	1	NC	1	3837.863	1
83		4 max	.002	1	.01	2	0	15	-4.346e-5	12	NC	1	NC	2
84		min	0	15	-.009	3	-.005	1	-1.009e-3	1	NC	1	4254.147	1
85		5 max	.001	1	.01	2	0	15	-4.346e-5	12	NC	1	NC	2
86		min	0	15	-.008	3	-.004	1	-1.009e-3	1	NC	1	4760.249	1
87		6 max	.001	1	.009	2	0	15	-4.346e-5	12	NC	1	NC	2
88		min	0	15	-.008	3	-.004	1	-1.009e-3	1	NC	1	5383.826	1
89		7 max	.001	1	.008	2	0	15	-4.346e-5	12	NC	1	NC	2
90		min	0	15	-.007	3	-.003	1	-1.009e-3	1	NC	1	6164.276	1
91		8 max	.001	1	.007	2	0	15	-4.346e-5	12	NC	1	NC	2
92		min	0	15	-.007	3	-.003	1	-1.009e-3	1	NC	1	7159.292	1
93		9 max	.001	1	.007	2	0	15	-4.346e-5	12	NC	1	NC	2
94		min	0	15	-.006	3	-.002	1	-1.009e-3	1	NC	1	8456.15	1
95		10 max	0	1	.006	2	0	15	-4.346e-5	12	NC	1	NC	1
96		min	0	15	-.005	3	-.002	1	-1.009e-3	1	NC	1	NC	1
97		11 max	0	1	.005	2	0	15	-4.346e-5	12	NC	1	NC	1
98		min	0	15	-.005	3	-.002	1	-1.009e-3	1	NC	1	NC	1
99		12 max	0	1	.005	2	0	15	-4.346e-5	12	NC	1	NC	1
100		min	0	15	-.004	3	-.001	1	-1.009e-3	1	NC	1	NC	1
101		13 max	0	1	.004	2	0	15	-4.346e-5	12	NC	1	NC	1
102		min	0	15	-.004	3	0	1	-1.009e-3	1	NC	1	NC	1
103		14 max	0	1	.003	2	0	15	-4.346e-5	12	NC	1	NC	1
104		min	0	15	-.003	3	0	1	-1.009e-3	1	NC	1	NC	1
105		15 max	0	1	.003	2	0	15	-4.346e-5	12	NC	1	NC	1
106		min	0	15	-.002	3	0	1	-1.009e-3	1	NC	1	NC	1
107		16 max	0	1	.002	2	0	15	-4.346e-5	12	NC	1	NC	1
108		min	0	15	-.002	3	0	1	-1.009e-3	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-4.346e-5	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-1.009e-3	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-4.346e-5	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-1.009e-3	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-4.346e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.009e-3	1	NC	1	NC	1
115	M6	1	max	.009	1	.038	2	.005	1	3.619e-4	3	NC	3	NC	2
116			min	-.012	3	-.035	3	-.004	3	8.892e-7	10	1130.202	2	8484.385	1
117		2	max	.008	1	.035	2	.005	1	3.496e-4	3	NC	3	NC	2
118			min	-.012	3	-.033	3	-.004	3	1.456e-7	10	1209.878	2	9197.586	1
119		3	max	.008	1	.033	2	.004	1	3.373e-4	3	NC	3	NC	1
120			min	-.011	3	-.031	3	-.004	3	-1.04e-6	2	1301.234	2	NC	1
121		4	max	.007	1	.03	2	.004	1	3.25e-4	3	NC	3	NC	1
122			min	-.01	3	-.029	3	-.004	3	-3.858e-6	2	1406.597	2	NC	1
123		5	max	.007	1	.028	2	.003	1	3.128e-4	3	NC	3	NC	1
124			min	-.01	3	-.027	3	-.003	3	-6.677e-6	2	1528.963	2	NC	1
125		6	max	.006	1	.025	2	.003	1	3.005e-4	3	NC	3	NC	1
126			min	-.009	3	-.026	3	-.003	3	-9.495e-6	2	1672.252	2	NC	1
127		7	max	.006	1	.023	2	.003	1	2.882e-4	3	NC	3	NC	1
128			min	-.008	3	-.024	3	-.003	3	-1.231e-5	2	1841.693	2	NC	1
129		8	max	.005	1	.021	2	.002	1	2.759e-4	3	NC	3	NC	1
130			min	-.007	3	-.022	3	-.002	3	-1.513e-5	2	2044.414	2	NC	1
131		9	max	.005	1	.019	2	.002	1	2.637e-4	3	NC	3	NC	1
132			min	-.007	3	-.02	3	-.002	3	-1.795e-5	2	2290.387	2	NC	1
133		10	max	.004	1	.016	2	.002	1	2.514e-4	3	NC	3	NC	1
134			min	-.006	3	-.018	3	-.002	3	-2.077e-5	2	2594.013	2	NC	1
135		11	max	.004	1	.014	2	.001	1	2.391e-4	3	NC	3	NC	1
136			min	-.005	3	-.016	3	-.002	3	-2.359e-5	2	2976.875	2	NC	1
137		12	max	.003	1	.012	2	.001	1	2.268e-4	3	NC	3	NC	1
138			min	-.005	3	-.014	3	-.001	3	-2.641e-5	2	3472.865	2	NC	1
139		13	max	.003	1	.01	2	0	1	2.146e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.001	3	-2.922e-5	2	4138.441	2	NC	1
141		14	max	.002	1	.008	2	0	1	2.023e-4	3	NC	3	NC	1
142			min	-.003	3	-.01	3	0	3	-3.204e-5	2	5075.169	2	NC	1
143		15	max	.002	1	.007	2	0	1	1.9e-4	3	NC	1	NC	1
144			min	-.003	3	-.008	3	0	3	-3.486e-5	2	6486.106	2	NC	1
145		16	max	.001	1	.005	2	0	1	1.777e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-3.768e-5	2	8844.924	2	NC	1
147		17	max	0	1	.003	2	0	1	1.655e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-4.05e-5	2	NC	1	NC	1
149		18	max	0	1	.002	2	0	1	1.532e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-4.332e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.409e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-5.349e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.522e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-6.711e-5	3	NC	1	NC	1
155		2	max	0	3	.002	2	0	3	2.347e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-4.88e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.172e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-3.049e-5	3	NC	1	NC	1
159		4	max	.001	3	.005	2	0	3	1.997e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.219e-5	3	9965.3	2	NC	1
161		5	max	.002	3	.006	2	.001	3	1.823e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	6.798e-7	15	7518.677	2	NC	1
163		6	max	.002	3	.008	2	.001	3	2.443e-5	3	NC	3	NC	1
164			min	-.002	2	-.01	3	0	1	7.425e-7	15	6021.548	2	NC	1
165		7	max	.003	3	.009	2	.002	3	4.274e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.012	3	0	1	8.051e-7	15	5001.43	2	NC	1
167		8	max	.003	3	.011	2	.002	3	6.105e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-1.682e-6	2	4256.307	2	NC	1
169		9	max	.003	3	.012	2	.002	3	7.936e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	-.001	1	-5.053e-6	2	3685.427	2	NC	1
171		10	max	.004	3	.014	2	.002	3	9.767e-5	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-8.425e-6	2	3232.9	2	NC	1
173		11	max	.004	3	.016	2	.002	3	1.16e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.18e-5	2	2865.17	2	NC	1
175		12	max	.005	3	.018	2	.002	3	1.343e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.002	1	-1.517e-5	2	2560.787	2	NC	1
177		13	max	.005	3	.02	2	.002	3	1.526e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.854e-5	2	2305.323	2	NC	1
179		14	max	.006	3	.022	2	.002	3	1.709e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-2.191e-5	2	2088.649	2	NC	1
181		15	max	.006	3	.024	2	.002	3	1.892e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.002	1	-2.528e-5	2	1903.395	2	NC	1
183		16	max	.006	3	.026	2	.002	3	2.075e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.002	1	-2.865e-5	2	1744.034	2	NC	1
185		17	max	.007	3	.029	2	.002	3	2.258e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.002	1	-3.203e-5	2	1606.321	2	NC	1
187		18	max	.007	3	.031	2	.002	3	2.441e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-3.54e-5	2	1486.927	2	NC	1
189		19	max	.008	3	.033	2	.002	3	2.625e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-3.877e-5	2	1383.202	2	NC	1
191	M8	1	max	.005	1	.044	2	.003	1	-6.195e-6	10	NC	1	NC	2
192			min	0	15	-.034	3	-.001	3	-2.128e-4	3	NC	1	7595.615	1
193		2	max	.004	1	.041	2	.002	1	-6.195e-6	10	NC	1	NC	2
194			min	0	15	-.032	3	-.001	3	-2.128e-4	3	NC	1	8281.229	1
195		3	max	.004	1	.039	2	.002	1	-6.195e-6	10	NC	1	NC	2
196			min	0	15	-.031	3	-.001	3	-2.128e-4	3	NC	1	9097.456	1
197		4	max	.004	1	.036	2	.002	1	-6.195e-6	10	NC	1	NC	1
198			min	0	15	-.029	3	-.001	3	-2.128e-4	3	NC	1	NC	1
199		5	max	.004	1	.034	2	.002	1	-6.195e-6	10	NC	1	NC	1
200			min	0	15	-.027	3	-.001	3	-2.128e-4	3	NC	1	NC	1
201		6	max	.003	1	.031	2	.002	1	-6.195e-6	10	NC	1	NC	1
202			min	0	15	-.025	3	0	3	-2.128e-4	3	NC	1	NC	1
203		7	max	.003	1	.029	2	.001	1	-6.195e-6	10	NC	1	NC	1
204			min	0	15	-.023	3	0	3	-2.128e-4	3	NC	1	NC	1
205		8	max	.003	1	.027	2	.001	1	-6.195e-6	10	NC	1	NC	1
206			min	0	15	-.021	3	0	3	-2.128e-4	3	NC	1	NC	1
207		9	max	.003	1	.024	2	0	1	-6.195e-6	10	NC	1	NC	1
208			min	0	15	-.019	3	0	3	-2.128e-4	3	NC	1	NC	1
209		10	max	.002	1	.022	2	0	1	-6.195e-6	10	NC	1	NC	1
210			min	0	15	-.017	3	0	3	-2.128e-4	3	NC	1	NC	1
211		11	max	.002	1	.019	2	0	1	-6.195e-6	10	NC	1	NC	1
212			min	0	15	-.015	3	0	3	-2.128e-4	3	NC	1	NC	1
213		12	max	.002	1	.017	2	0	1	-6.195e-6	10	NC	1	NC	1
214			min	0	15	-.013	3	0	3	-2.128e-4	3	NC	1	NC	1
215		13	max	.002	1	.015	2	0	1	-6.195e-6	10	NC	1	NC	1
216			min	0	15	-.011	3	0	3	-2.128e-4	3	NC	1	NC	1
217		14	max	.001	1	.012	2	0	1	-6.195e-6	10	NC	1	NC	1
218			min	0	15	-.01	3	0	3	-2.128e-4	3	NC	1	NC	1
219		15	max	.001	1	.01	2	0	1	-6.195e-6	10	NC	1	NC	1
220			min	0	15	-.008	3	0	3	-2.128e-4	3	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-6.195e-6	10	NC	1	NC	1
222			min	0	15	-.006	3	0	3	-2.128e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.005	2	0	1	-6.195e-6	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.128e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-6.195e-6	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.128e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-6.195e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.128e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.01	2	0	3	1.124e-3	1	NC	3	NC	1
230			min	-.003	3	-.011	3	-.002	1	-3.506e-4	3	4086.647	2	NC	1
231		2	max	.003	1	.009	2	0	3	1.066e-3	1	NC	3	NC	1
232			min	-.003	3	-.01	3	-.002	1	-3.388e-4	3	4474.201	2	NC	1
233		3	max	.003	1	.009	2	0	3	1.008e-3	1	NC	1	NC	1
234			min	-.003	3	-.01	3	-.002	1	-3.27e-4	3	4937.801	2	NC	1
235		4	max	.002	1	.008	2	0	3	9.498e-4	1	NC	1	NC	1
236			min	-.003	3	-.01	3	-.002	1	-3.152e-4	3	5496.27	2	NC	1
237		5	max	.002	1	.007	2	0	3	8.915e-4	1	NC	1	NC	1
238			min	-.003	3	-.009	3	-.002	1	-3.034e-4	3	6174.835	2	NC	1
239		6	max	.002	1	.006	2	0	3	8.333e-4	1	NC	1	NC	1
240			min	-.003	3	-.009	3	-.002	1	-2.916e-4	3	7007.862	2	NC	1
241		7	max	.002	1	.005	2	0	3	7.751e-4	1	NC	1	NC	1
242			min	-.002	3	-.008	3	-.002	1	-2.798e-4	3	8043.067	2	NC	1
243		8	max	.002	1	.005	2	0	3	7.168e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	-.002	1	-2.68e-4	3	9348.189	2	NC	1
245		9	max	.002	1	.004	2	0	3	6.586e-4	1	NC	1	NC	1
246			min	-.002	3	-.007	3	-.001	1	-2.563e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	6.003e-4	1	NC	1	NC	1
248			min	-.002	3	-.007	3	-.001	1	-2.445e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	5.421e-4	1	NC	1	NC	1
250			min	-.002	3	-.006	3	-.001	1	-2.327e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.839e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-2.209e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	4.256e-4	1	NC	1	NC	1
254			min	-.001	3	-.005	3	0	1	-2.091e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.674e-4	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-1.973e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	3.092e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-1.855e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.509e-4	1	NC	1	NC	1
260			min	0	3	-.003	3	0	1	-1.737e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.927e-4	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-1.619e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.345e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.501e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	7.623e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.383e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	6.613e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.762e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	4.629e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-8.996e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	11	2.645e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.423e-4	1	NC	1	NC	1
273		4	max	0	3	0	2	0	10	6.614e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.946e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	-9.084e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-2.47e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	15	-1.359e-5	15	NC	1	NC	1
278			min	0	2	-.005	3	-.001	3	-2.993e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	15	-1.61e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	1	-3.516e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	15	-1.861e-5	15	NC	1	NC	1
282			min	0	2	-.006	3	-.003	1	-4.04e-4	1	NC	1	NC	1
283		9	max	.001	3	.001	2	0	15	-2.112e-5	15	NC	1	NC	1
284			min	0	2	-.007	3	-.004	1	-4.563e-4	1	NC	1	NC	1
285		10	max	.001	3	.002	2	0	15	-2.362e-5	15	NC	1	NC	2
286			min	-.001	2	-.007	3	-.005	1	-5.086e-4	1	NC	1	9758.443	1
287		11	max	.001	3	.002	2	0	15	-2.613e-5	15	NC	1	NC	2
288			min	-.001	2	-.008	3	-.006	1	-5.61e-4	1	NC	1	7945.959	1
289		12	max	.001	3	.003	2	0	15	-2.864e-5	15	NC	1	NC	2
290			min	-.001	2	-.008	3	-.007	1	-6.133e-4	1	NC	1	6643.196	1
291		13	max	.002	3	.004	2	0	15	-3.115e-5	15	NC	1	NC	2
292			min	-.001	2	-.008	3	-.008	1	-6.657e-4	1	NC	1	5674.827	1
293		14	max	.002	3	.004	2	0	15	-3.366e-5	15	NC	1	NC	2
294			min	-.002	2	-.009	3	-.009	1	-7.18e-4	1	NC	1	4935.523	1
295		15	max	.002	3	.005	2	0	15	-3.617e-5	15	NC	1	NC	2
296			min	-.002	2	-.009	3	-.011	1	-7.703e-4	1	8816.236	2	4358.823	1
297		16	max	.002	3	.006	2	0	15	-3.868e-5	15	NC	1	NC	2
298			min	-.002	2	-.009	3	-.012	1	-8.227e-4	1	7458.15	2	3901.072	1
299		17	max	.002	3	.007	2	0	15	-4.118e-5	15	NC	1	NC	3
300			min	-.002	2	-.009	3	-.013	1	-8.75e-4	1	6410.806	2	3532.635	1
301		18	max	.002	3	.008	2	0	15	-4.369e-5	15	NC	1	NC	3
302			min	-.002	2	-.009	3	-.014	1	-9.273e-4	1	5593.205	2	3232.825	1
303		19	max	.002	3	.009	2	0	15	-4.62e-5	15	NC	3	NC	3
304			min	-.002	2	-.009	3	-.015	1	-9.797e-4	1	4948.827	2	2986.874	1
305	M12	1	max	.002	1	.012	2	.013	1	9.58e-4	1	NC	1	NC	3
306			min	0	15	-.011	3	0	15	4.61e-5	15	NC	1	1515.768	1
307		2	max	.002	1	.012	2	.012	1	9.58e-4	1	NC	1	NC	3
308			min	0	15	-.01	3	0	15	4.61e-5	15	NC	1	1653.08	1
309		3	max	.002	1	.011	2	.011	1	9.58e-4	1	NC	1	NC	3
310			min	0	15	-.01	3	0	15	4.61e-5	15	NC	1	1816.527	1
311		4	max	.002	1	.01	2	.01	1	9.58e-4	1	NC	1	NC	3
312			min	0	15	-.009	3	0	15	4.61e-5	15	NC	1	2012.998	1
313		5	max	.001	1	.01	2	.009	1	9.58e-4	1	NC	1	NC	3
314			min	0	15	-.008	3	0	15	4.61e-5	15	NC	1	2251.876	1
315		6	max	.001	1	.009	2	.008	1	9.58e-4	1	NC	1	NC	3
316			min	0	15	-.008	3	0	15	4.61e-5	15	NC	1	2546.216	1
317		7	max	.001	1	.008	2	.007	1	9.58e-4	1	NC	1	NC	3
318			min	0	15	-.007	3	0	15	4.61e-5	15	NC	1	2914.611	1
319		8	max	.001	1	.007	2	.006	1	9.58e-4	1	NC	1	NC	3
320			min	0	15	-.007	3	0	15	4.61e-5	15	NC	1	3384.289	1
321		9	max	.001	1	.007	2	.005	1	9.58e-4	1	NC	1	NC	2
322			min	0	15	-.006	3	0	15	4.61e-5	15	NC	1	3996.435	1
323		10	max	0	1	.006	2	.004	1	9.58e-4	1	NC	1	NC	2
324			min	0	15	-.005	3	0	15	4.61e-5	15	NC	1	4815.804	1
325		11	max	0	1	.005	2	.003	1	9.58e-4	1	NC	1	NC	2
326			min	0	15	-.005	3	0	15	4.61e-5	15	NC	1	5949.053	1
327		12	max	0	1	.005	2	.003	1	9.58e-4	1	NC	1	NC	2
328			min	0	15	-.004	3	0	15	4.61e-5	15	NC	1	7582.251	1
329		13	max	0	1	.004	2	.002	1	9.58e-4	1	NC	1	NC	1
330			min	0	15	-.004	3	0	15	4.61e-5	15	NC	1	NC	1
331		14	max	0	1	.003	2	.001	1	9.58e-4	1	NC	1	NC	1
332			min	0	15	-.003	3	0	15	4.61e-5	15	NC	1	NC	1
333		15	max	0	1	.003	2	0	1	9.58e-4	1	NC	1	NC	1
334			min	0	15	-.002	3	0	15	4.61e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	9.58e-4	1	NC	1	NC	1
336			min	0	15	-.002	3	0	15	4.61e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	9.58e-4	1	NC	1	NC	1
338			min	0	15	-.001	3	0	15	4.61e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	9.58e-4	1	NC	1	NC	1
340			min	0	15	0	3	0	15	4.61e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	9.58e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	4.61e-5	15	NC	1	NC	1
343	M1	1	max	.009	3	.026	3	.002	3	1.853e-2	1	NC	1	NC	1
344			min	-.009	2	-.024	2	-.005	1	-2.414e-2	3	NC	1	NC	1
345		2	max	.009	3	.016	3	.002	3	8.776e-3	1	NC	4	NC	2
346			min	-.009	2	-.014	2	-.011	1	-1.196e-2	3	4924.142	2	7644.944	1
347		3	max	.009	3	.007	3	0	3	-1.225e-5	12	NC	4	NC	2
348			min	-.009	2	-.005	2	-.015	1	-7.915e-4	1	2527.671	2	4638.269	1
349		4	max	.009	3	.003	1	0	3	-7.824e-6	12	NC	4	NC	3
350			min	-.009	2	-.002	3	-.017	1	-6.845e-4	1	1768.448	2	3840.809	1
351		5	max	.009	3	.01	2	0	3	-1.578e-6	3	NC	4	NC	3
352			min	-.009	2	-.008	3	-.018	1	-5.775e-4	1	1401.684	2	3691.663	1
353		6	max	.009	3	.016	2	0	3	5.e-6	3	NC	5	NC	2
354			min	-.009	2	-.014	3	-.017	1	-4.705e-4	1	1192.921	2	3955.753	1
355		7	max	.009	3	.02	2	0	3	1.158e-5	3	NC	5	NC	2
356			min	-.009	2	-.018	3	-.015	1	-3.635e-4	1	1065.135	2	4720.89	1
357		8	max	.009	3	.024	2	0	3	1.816e-5	3	NC	5	NC	2
358			min	-.009	2	-.021	3	-.012	1	-2.566e-4	1	986.266	2	6508.925	1
359		9	max	.009	3	.026	2	0	3	2.474e-5	3	NC	5	NC	1
360			min	-.009	2	-.023	3	-.008	1	-1.496e-4	1	941.363	2	NC	1
361		10	max	.009	3	.027	2	0	3	3.131e-5	3	NC	5	NC	1
362			min	-.009	2	-.023	3	-.005	1	-4.26e-5	1	923.609	2	NC	1
363		11	max	.009	3	.027	2	0	3	6.438e-5	1	NC	5	NC	1
364			min	-.009	2	-.022	3	-.001	1	3.354e-6	15	931.146	2	NC	1
365		12	max	.009	3	.025	2	.002	1	1.714e-4	1	NC	5	NC	2
366			min	-.009	2	-.021	3	0	15	8.215e-6	15	966.419	2	7252.262	1
367		13	max	.009	3	.022	2	.005	1	2.784e-4	1	NC	5	NC	2
368			min	-.009	2	-.018	3	0	15	1.308e-5	15	1037.498	2	5073.349	1
369		14	max	.009	3	.017	2	.007	1	3.853e-4	1	NC	5	NC	2
370			min	-.009	2	-.013	3	0	15	1.794e-5	15	1162.786	2	4174.946	1
371		15	max	.009	3	.01	2	.008	1	4.923e-4	1	NC	4	NC	3
372			min	-.009	2	-.008	3	0	15	2.28e-5	15	1385.218	2	3854.094	1
373		16	max	.009	3	.002	1	.008	1	5.649e-4	1	NC	4	NC	3
374			min	-.009	2	-.002	3	0	15	2.612e-5	15	1824.001	2	3978.836	1
375		17	max	.009	3	.006	3	.006	1	2.785e-5	3	NC	4	NC	2
376			min	-.009	2	-.008	2	0	15	-1.821e-4	1	2573.052	1	4779.874	1
377		18	max	.009	3	.014	3	.002	1	1.286e-2	2	NC	2	NC	2
378			min	-.009	2	-.019	2	0	15	-5.95e-3	3	4973.31	1	7850.126	1
379		19	max	.009	3	.022	3	0	3	2.604e-2	2	NC	1	NC	1
380			min	-.009	2	-.032	2	-.004	1	-1.204e-2	3	5950.696	2	NC	1
381	M5	1	max	.03	3	.086	3	.002	3	1.914e-6	3	NC	1	NC	1
382			min	-.034	2	-.08	2	-.006	1	5.091e-8	10	3560.161	3	NC	1
383		2	max	.03	3	.052	3	.003	3	1.004e-4	3	NC	5	NC	1
384			min	-.034	2	-.048	2	-.006	1	-5.483e-5	1	1465.845	2	NC	1
385		3	max	.03	3	.021	3	.004	3	1.97e-4	3	NC	5	NC	1
386			min	-.034	2	-.018	2	-.005	1	-1.095e-4	1	751.991	2	NC	1
387		4	max	.03	3	.009	2	.005	3	1.895e-4	3	NC	5	NC	1
388			min	-.034	2	-.005	3	-.004	1	-1.048e-4	1	525.553	2	NC	1
389		5	max	.03	3	.033	2	.006	3	1.82e-4	3	NC	5	NC	1
390			min	-.034	2	-.027	3	-.004	1	-1.002e-4	1	416.118	2	NC	1
391		6	max	.03	3	.053	2	.006	3	1.746e-4	3	NC	15	NC	1
392			min	-.034	2	-.045	3	-.004	1	-9.556e-5	1	353.794	2	NC	1
393		7	max	.029	3	.069	2	.006	3	1.671e-4	3	NC	15	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.034	2	-.059	3	-.003	1	-9.093e-5	1	315.612	2	NC	1
395		8	max	.029	3	.081	2	.006	3	1.596e-4	3	NC	15	NC	1
396			min	-.034	2	-.068	3	-.003	1	-8.629e-5	1	292.009	2	NC	1
397		9	max	.029	3	.088	2	.006	3	1.521e-4	3	NC	15	NC	1
398			min	-.034	2	-.074	3	-.003	1	-8.165e-5	1	278.522	2	NC	1
399		10	max	.029	3	.092	2	.005	3	1.446e-4	3	NC	15	NC	1
400			min	-.034	2	-.075	3	-.003	1	-7.702e-5	1	273.114	2	NC	1
401		11	max	.029	3	.09	2	.005	3	1.371e-4	3	NC	15	NC	1
402			min	-.034	2	-.073	3	-.003	1	-7.238e-5	1	275.227	2	NC	1
403		12	max	.029	3	.084	2	.005	3	1.297e-4	3	NC	15	NC	1
404			min	-.034	2	-.067	3	-.002	1	-6.775e-5	1	285.585	2	NC	1
405		13	max	.029	3	.073	2	.004	3	1.222e-4	3	NC	15	NC	1
406			min	-.034	2	-.057	3	-.002	1	-6.311e-5	1	306.593	2	NC	1
407		14	max	.029	3	.056	2	.003	3	1.147e-4	3	NC	5	NC	1
408			min	-.034	2	-.043	3	-.002	1	-5.847e-5	1	343.754	2	NC	1
409		15	max	.029	3	.034	2	.003	3	1.072e-4	3	NC	5	NC	1
410			min	-.034	2	-.026	3	-.002	1	-5.384e-5	1	409.953	2	NC	1
411		16	max	.028	3	.007	1	.002	3	9.514e-5	3	NC	5	NC	1
412			min	-.034	2	-.006	3	-.002	1	-5.733e-5	1	541.216	2	NC	1
413		17	max	.028	3	.019	3	.002	3	-1.08e-5	15	NC	5	NC	1
414			min	-.034	2	-.027	2	-.003	1	-2.543e-4	1	868.16	3	NC	1
415		18	max	.028	3	.045	3	0	3	-5.551e-6	15	NC	5	NC	1
416			min	-.034	2	-.066	2	-.003	1	-1.302e-4	1	1702.75	3	NC	1
417		19	max	.028	3	.073	3	0	3	-3.899e-8	15	NC	3	NC	1
418			min	-.034	2	-.107	2	-.003	1	-3.901e-7	3	1707.929	2	NC	1
419	M9	1	max	.009	3	.026	3	.002	3	2.414e-2	3	NC	1	NC	1
420			min	-.009	2	-.024	2	-.007	1	-1.852e-2	1	NC	1	NC	1
421		2	max	.009	3	.016	3	0	3	1.193e-2	3	NC	4	NC	2
422			min	-.009	2	-.014	2	-.001	1	-9.015e-3	1	4925.585	2	8657.882	1
423		3	max	.009	3	.006	3	.002	1	3.151e-4	1	NC	4	NC	2
424			min	-.009	2	-.005	2	0	3	-5.6e-5	3	2528.431	2	5355.385	1
425		4	max	.009	3	.003	2	.004	1	2.247e-4	1	NC	4	NC	2
426			min	-.009	2	-.002	3	0	3	-6.122e-5	3	1768.98	2	4521.531	1
427		5	max	.009	3	.01	2	.005	1	1.343e-4	1	NC	4	NC	3
428			min	-.009	2	-.009	3	-.002	3	-6.644e-5	3	1402.087	2	4459.803	1
429		6	max	.009	3	.016	2	.004	1	4.391e-5	1	NC	5	NC	2
430			min	-.009	2	-.014	3	-.002	3	-7.166e-5	3	1193.241	2	4968.038	1
431		7	max	.009	3	.02	2	.002	1	5.894e-6	10	NC	5	NC	2
432			min	-.009	2	-.018	3	-.003	3	-7.688e-5	3	1065.396	2	6337.53	1
433		8	max	.009	3	.024	2	0	2	-4.407e-6	10	NC	5	NC	1
434			min	-.009	2	-.021	3	-.003	3	-1.369e-4	1	986.481	2	NC	1
435		9	max	.009	3	.026	2	0	10	-1.052e-5	15	NC	5	NC	1
436			min	-.009	2	-.023	3	-.004	3	-2.273e-4	1	941.538	2	9562.301	3
437		10	max	.009	3	.027	2	0	15	-1.462e-5	15	NC	5	NC	1
438			min	-.009	2	-.023	3	-.006	1	-3.177e-4	1	923.744	2	9278.352	3
439		11	max	.009	3	.027	2	0	15	-1.872e-5	15	NC	5	NC	2
440			min	-.009	2	-.023	3	-.01	1	-4.081e-4	1	931.239	2	8789.787	1
441		12	max	.009	3	.025	2	0	15	-2.283e-5	15	NC	5	NC	2
442			min	-.009	2	-.021	3	-.012	1	-4.985e-4	1	966.459	2	5585.059	1
443		13	max	.009	3	.022	2	0	15	-2.693e-5	15	NC	5	NC	2
444			min	-.009	2	-.018	3	-.015	1	-5.889e-4	1	1037.46	2	4301.343	1
445		14	max	.009	3	.017	2	0	15	-3.103e-5	15	NC	5	NC	2
446			min	-.009	2	-.013	3	-.016	1	-6.793e-4	1	1162.616	2	3721.939	1
447		15	max	.009	3	.01	2	0	15	-3.514e-5	15	NC	4	NC	3
448			min	-.009	2	-.008	3	-.016	1	-7.697e-4	1	1384.781	2	3543.29	1
449		16	max	.009	3	.002	1	0	15	-3.797e-5	15	NC	4	NC	2
450			min	-.009	2	-.002	3	-.015	1	-8.336e-4	1	1822.862	2	3734.21	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.009	3	.006	3	0	15	6.864e-5	3	NC	4	NC	2
452			min	-.009	2	-.008	2	-.013	1	-2.667e-4	1	2572.856	1	4552.407	1
453		18	max	.009	3	.014	3	0	15	6.e-3	3	NC	2	NC	2
454			min	-.009	2	-.019	2	-.008	1	-1.296e-2	2	4972.944	1	7557.78	1
455		19	max	.009	3	.023	3	0	3	1.204e-2	3	NC	1	NC	1
456			min	-.009	2	-.031	2	-.002	1	-2.604e-2	2	5976.384	2	NC	1
457	M13	1	max	.007	1	.026	3	.009	3	3.87e-3	3	NC	1	NC	1
458			min	-.002	3	-.024	2	-.009	2	-3.614e-3	2	NC	1	NC	1
459		2	max	.007	1	.251	3	.044	1	4.849e-3	3	NC	5	NC	3
460			min	-.002	3	-.194	1	0	10	-4.561e-3	2	772.062	3	3597.31	1
461		3	max	.007	1	.436	3	.112	1	5.828e-3	3	NC	5	NC	3
462			min	-.002	3	-.335	1	.005	15	-5.509e-3	2	424.903	3	1492.342	1
463		4	max	.007	1	.551	3	.17	1	6.807e-3	3	NC	15	NC	3
464			min	-.002	3	-.424	1	.008	15	-6.457e-3	2	331.669	3	996.947	1
465		5	max	.007	1	.584	3	.199	1	7.786e-3	3	NC	15	NC	3
466			min	-.002	3	-.45	1	.009	15	-7.404e-3	2	312.046	3	857.708	1
467		6	max	.007	1	.536	3	.189	1	8.765e-3	3	NC	15	NC	3
468			min	-.002	3	-.415	1	.009	15	-8.352e-3	2	341.141	3	901.05	1
469		7	max	.006	1	.424	3	.143	1	9.744e-3	3	NC	5	NC	5
470			min	-.002	3	-.33	1	.004	10	-9.3e-3	2	436.959	3	1178.865	1
471		8	max	.006	1	.279	3	.075	1	1.072e-2	3	NC	5	NC	5
472			min	-.002	3	-.22	1	-.006	10	-1.025e-2	2	687.727	3	2188.392	1
473		9	max	.006	1	.146	3	.028	3	1.17e-2	3	NC	5	NC	1
474			min	-.002	3	-.124	2	-.02	2	-1.12e-2	2	1449.093	3	9401.507	3
475		10	max	.006	1	.086	3	.03	3	1.268e-2	3	NC	4	NC	4
476			min	-.002	3	-.08	2	-.034	2	-1.214e-2	2	2914.107	3	7111.719	2
477		11	max	.006	1	.146	3	.034	3	1.17e-2	3	NC	5	NC	1
478			min	-.002	3	-.124	2	-.02	2	-1.12e-2	2	1449.091	3	6985.165	3
479		12	max	.006	1	.279	3	.081	1	1.072e-2	3	NC	5	NC	5
480			min	-.002	3	-.22	1	-.006	10	-1.025e-2	2	687.727	3	2050.703	1
481		13	max	.006	1	.424	3	.15	1	9.747e-3	3	NC	5	NC	5
482			min	-.002	3	-.33	1	.004	10	-9.3e-3	2	436.959	3	1131.103	1
483		14	max	.006	1	.536	3	.195	1	8.769e-3	3	NC	15	NC	5
484			min	-.002	3	-.415	1	.009	15	-8.353e-3	2	341.141	3	872.917	1
485		15	max	.006	1	.584	3	.204	1	7.791e-3	3	NC	15	NC	5
486			min	-.002	3	-.45	1	.01	15	-7.405e-3	2	312.046	3	834.979	1
487		16	max	.006	1	.551	3	.175	1	6.813e-3	3	NC	15	NC	5
488			min	-.002	3	-.424	1	.008	15	-6.457e-3	2	331.668	3	972.765	1
489		17	max	.006	1	.436	3	.115	1	5.835e-3	3	NC	5	NC	3
490			min	-.002	3	-.335	1	.006	15	-5.51e-3	2	424.903	3	1456.325	1
491		18	max	.005	1	.252	3	.046	1	4.857e-3	3	NC	5	NC	3
492			min	-.002	3	-.194	1	0	10	-4.562e-3	2	772.062	3	3498.424	1
493		19	max	.005	1	.026	3	.009	3	3.879e-3	3	NC	1	NC	1
494			min	-.002	3	-.024	2	-.009	2	-3.615e-3	2	NC	1	NC	1
495	M16	1	max	.002	1	.023	3	.009	3	4.616e-3	2	NC	1	NC	1
496			min	0	3	-.031	2	-.009	2	-3.24e-3	3	NC	1	NC	1
497		2	max	.002	1	.135	3	.046	1	5.841e-3	2	NC	5	NC	3
498			min	0	3	-.275	2	0	10	-4.045e-3	3	715.483	2	3421.31	1
499		3	max	.002	1	.227	3	.116	1	7.067e-3	2	NC	5	NC	3
500			min	0	3	-.474	2	.006	15	-4.849e-3	3	393.485	2	1439.645	1
501		4	max	.002	1	.286	3	.175	1	8.292e-3	2	NC	15	NC	5
502			min	0	3	-.599	2	.008	15	-5.654e-3	3	306.748	2	967.163	1
503		5	max	.002	1	.305	3	.204	1	9.517e-3	2	NC	15	NC	5
504			min	0	3	-.636	2	.01	15	-6.458e-3	3	287.966	2	833.689	1
505		6	max	.002	1	.285	3	.194	1	1.074e-2	2	NC	15	NC	5
506			min	0	3	-.586	2	.009	15	-7.263e-3	3	313.611	2	875.155	1
507		7	max	.002	1	.233	3	.148	1	1.197e-2	2	NC	5	NC	5



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-468	2	.005	10	-8.067e-3	3	398.751	2	1140.106	1
509	8	max	.003	1	.165	3	.079	1	1.319e-2	2	NC	5	NC	5
510		min	0	3	-.313	2	-.005	10	-8.872e-3	3	617.223	2	2088.761	1
511	9	max	.003	1	.102	3	.032	3	1.442e-2	2	NC	5	NC	1
512		min	0	3	-.172	2	-.02	2	-9.676e-3	3	1240.746	2	7613.91	3
513	10	max	.003	1	.073	3	.028	3	1.564e-2	2	NC	4	NC	4
514		min	0	3	-.107	2	-.034	2	-1.048e-2	3	2294.212	2	7168.635	2
515	11	max	.003	1	.102	3	.028	3	1.442e-2	2	NC	5	NC	1
516		min	0	3	-.172	2	-.02	2	-9.675e-3	3	1240.746	2	8980.524	3
517	12	max	.003	1	.165	3	.077	1	1.319e-2	2	NC	5	NC	5
518		min	0	3	-.313	2	-.005	10	-8.869e-3	3	617.223	2	2130.928	1
519	13	max	.003	1	.233	3	.146	1	1.197e-2	2	NC	5	NC	5
520		min	0	3	-468	2	.005	10	-8.064e-3	3	398.751	2	1158.771	1
521	14	max	.003	1	.285	3	.191	1	1.074e-2	2	NC	15	NC	5
522		min	0	3	-.586	2	.009	15	-7.258e-3	3	313.611	2	889.053	1
523	15	max	.003	1	.305	3	.201	1	9.518e-3	2	NC	15	NC	5
524		min	0	3	-.636	2	.01	15	-6.453e-3	3	287.966	2	847.845	1
525	16	max	.003	1	.286	3	.172	1	8.293e-3	2	NC	15	NC	3
526		min	0	3	-.599	2	.008	15	-5.647e-3	3	306.748	2	986.214	1
527	17	max	.003	1	.227	3	.113	1	7.068e-3	2	NC	5	NC	3
528		min	0	3	-.474	2	.005	15	-4.842e-3	3	393.485	2	1475.87	1
529	18	max	.003	1	.135	3	.044	1	5.843e-3	2	NC	5	NC	3
530		min	0	3	-.275	2	0	10	-4.036e-3	3	715.483	2	3550.159	1
531	19	max	.004	1	.022	3	.009	3	4.618e-3	2	NC	1	NC	1
532		min	0	3	-.032	2	-.009	2	-3.231e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.97e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.991e-5	2	NC	1	NC	1
535	2	max	0	3	-.004	15	.001	1	9.16e-4	3	NC	5	NC	1
536		min	0	10	-.019	4	0	3	-6.025e-4	2	5183.539	4	NC	1
537	3	max	0	3	-.009	15	.004	1	1.435e-3	3	NC	15	NC	1
538		min	0	10	-.037	4	-.004	3	-1.145e-3	2	2637.726	4	NC	1
539	4	max	0	3	-.013	15	.008	1	1.954e-3	3	7698.444	15	NC	4
540		min	0	10	-.054	4	-.008	3	-1.688e-3	2	1809.634	4	6863.476	3
541	5	max	0	3	-.016	15	.013	1	2.473e-3	3	6007.174	15	NC	4
542		min	0	10	-.069	4	-.013	3	-2.23e-3	2	1412.076	4	4526.864	3
543	6	max	0	3	-.019	15	.018	1	2.992e-3	3	5055.672	15	NC	4
544		min	0	10	-.082	4	-.019	3	-2.773e-3	2	1188.411	4	3307.339	3
545	7	max	0	3	-.022	15	.023	1	3.511e-3	3	4483.469	15	NC	4
546		min	0	10	-.092	4	-.025	3	-3.315e-3	2	1053.906	4	2591.881	3
547	8	max	0	3	-.023	15	.028	1	4.03e-3	3	4140.061	15	NC	4
548		min	0	10	-.1	4	-.031	3	-3.858e-3	2	973.183	4	2141.104	3
549	9	max	0	3	-.025	15	.033	1	4.549e-3	3	3955.218	15	NC	4
550		min	0	10	-.105	4	-.036	3	-4.4e-3	2	929.733	4	1845.693	3
551	10	max	0	3	-.025	15	.036	1	5.068e-3	3	3896.743	15	NC	4
552		min	0	10	-.106	4	-.04	3	-4.943e-3	2	915.988	4	1650.607	3
553	11	max	0	3	-.025	15	.039	1	5.587e-3	3	3955.218	15	NC	5
554		min	0	10	-.105	4	-.043	3	-5.485e-3	2	929.733	4	1526.917	3
555	12	max	0	3	-.024	15	.039	1	6.106e-3	3	4140.061	15	NC	5
556		min	0	10	-.1	4	-.044	3	-6.028e-3	2	973.183	4	1460.3	3
557	13	max	.001	3	-.022	15	.038	1	6.625e-3	3	4483.469	15	NC	5
558		min	0	10	-.093	4	-.042	3	-6.571e-3	2	1053.906	4	1446.708	3
559	14	max	.001	3	-.019	15	.035	1	7.143e-3	3	5055.672	15	NC	4
560		min	0	10	-.082	4	-.038	3	-7.113e-3	2	1188.411	4	1492.595	3
561	15	max	.001	3	-.016	15	.029	1	7.662e-3	3	6007.174	15	NC	4
562		min	-.001	10	-.07	4	-.031	3	-7.656e-3	2	1412.076	4	1621.276	3
563	16	max	.001	3	-.013	15	.02	1	8.181e-3	3	7698.444	15	NC	4
564		min	-.001	10	-.055	4	-.02	3	-8.198e-3	2	1809.634	4	1895.899	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	-.009	15	.008	1	8.7e-3	3	NC	15	NC	4
566		min	-.001	10	-.038	4	-.006	3	-8.741e-3	2	2637.726	4	2514.451	3
567	18	max	.001	3	-.005	15	.013	3	9.219e-3	3	NC	5	NC	4
568		min	-.001	10	-.02	4	-.016	2	-9.283e-3	2	5183.539	4	4478.342	3
569	19	max	.002	3	.006	2	.037	3	9.738e-3	3	NC	1	NC	1
570		min	-.001	10	-.003	9	-.038	2	-9.826e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.011	3	2.869e-3	3	NC	1	NC	1
572		min	-.001	3	-.002	9	-.011	2	-2.689e-3	2	NC	1	NC	1
573	2	max	0	10	-.005	15	.004	9	2.756e-3	3	NC	5	NC	2
574		min	-.001	3	-.02	4	-.002	10	-2.572e-3	2	5183.539	4	9769.173	1
575	3	max	0	10	-.009	15	.012	1	2.642e-3	3	NC	15	NC	4
576		min	-.001	3	-.037	4	-.003	3	-2.456e-3	2	2637.726	4	5521.55	1
577	4	max	0	10	-.013	15	.018	1	2.529e-3	3	7698.444	15	NC	4
578		min	-.001	3	-.054	4	-.008	3	-2.339e-3	2	1809.634	4	4194.499	1
579	5	max	0	10	-.016	15	.022	1	2.416e-3	3	6007.174	15	NC	4
580		min	-.001	3	-.069	4	-.011	3	-2.222e-3	2	1412.076	4	3617.556	1
581	6	max	0	10	-.019	15	.024	1	2.303e-3	3	5055.672	15	NC	4
582		min	-.001	3	-.082	4	-.013	3	-2.106e-3	2	1188.411	4	3363.091	1
583	7	max	0	10	-.022	15	.025	1	2.189e-3	3	4483.469	15	NC	4
584		min	0	3	-.092	4	-.014	3	-1.989e-3	2	1053.906	4	3296.794	1
585	8	max	0	10	-.023	15	.025	1	2.076e-3	3	4140.061	15	NC	4
586		min	0	3	-.1	4	-.014	3	-1.872e-3	2	973.183	4	3372.245	1
587	9	max	0	10	-.025	15	.024	1	1.963e-3	3	3955.218	15	NC	4
588		min	0	3	-.105	4	-.013	3	-1.756e-3	2	929.733	4	3582.228	1
589	10	max	0	10	-.025	15	.021	1	1.85e-3	3	3896.743	15	NC	4
590		min	0	3	-.106	4	-.012	3	-1.639e-3	2	915.988	4	3947.144	1
591	11	max	0	10	-.025	15	.019	1	1.736e-3	3	3955.218	15	NC	4
592		min	0	3	-.105	4	-.01	3	-1.522e-3	2	929.733	4	4519.079	1
593	12	max	0	10	-.023	15	.015	1	1.623e-3	3	4140.061	15	NC	4
594		min	0	3	-.1	4	-.008	3	-1.406e-3	2	973.183	4	5401.462	1
595	13	max	0	10	-.022	15	.012	1	1.51e-3	3	4483.469	15	NC	3
596		min	0	3	-.092	4	-.006	3	-1.289e-3	2	1053.906	4	6799.438	1
597	14	max	0	10	-.019	15	.009	1	1.397e-3	3	5055.672	15	NC	2
598		min	0	3	-.082	4	-.004	3	-1.172e-3	2	1188.411	4	9152.315	1
599	15	max	0	10	-.016	15	.006	1	1.283e-3	3	6007.174	15	NC	1
600		min	0	3	-.069	4	-.002	3	-1.056e-3	2	1412.076	4	NC	1
601	16	max	0	10	-.013	15	.003	1	1.17e-3	3	7698.444	15	NC	1
602		min	0	3	-.054	4	0	3	-9.392e-4	2	1809.634	4	NC	1
603	17	max	0	10	-.009	15	.001	9	1.057e-3	3	NC	15	NC	1
604		min	0	3	-.037	4	0	2	-8.226e-4	2	2637.726	4	NC	1
605	18	max	0	10	-.004	15	0	3	9.437e-4	3	NC	5	NC	1
606		min	0	3	-.019	4	0	2	-7.059e-4	2	5183.539	4	NC	1
607	19	max	0	1	0	1	0	1	8.305e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.893e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

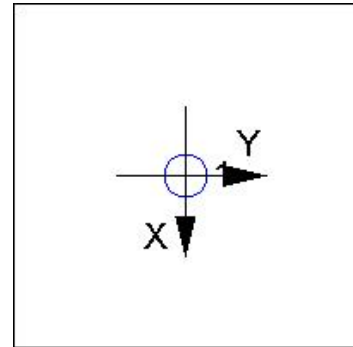
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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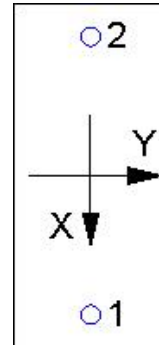
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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.