

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

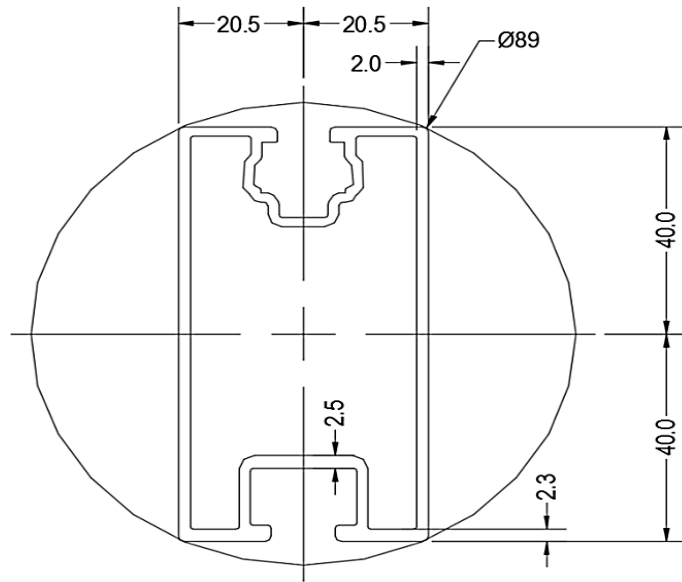
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

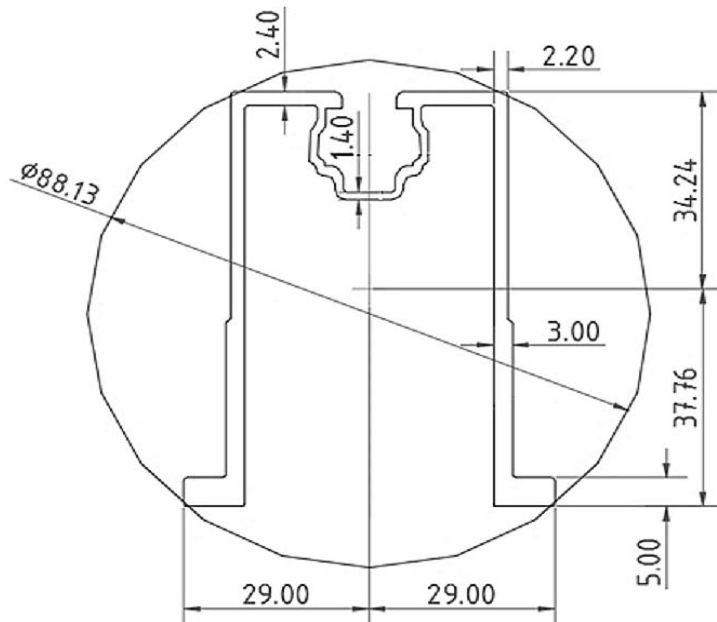
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	99 in
ΦF_{ty} STRONG-AXIS =	28.68 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.289 k-ft
M_z =	0.183 k-ft
$M_{y \text{ allowable}}$ =	1.782 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	94%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.66 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.647 k-ft
M_z =	0.000 k-ft
P_n =	0.203 k
$M_{y \text{ allowable}}$ =	1.455 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	46%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.752 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	14%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.164 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	4%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.408 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<u>17%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.028 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>13%</u>



A cross brace kit is required every 14 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

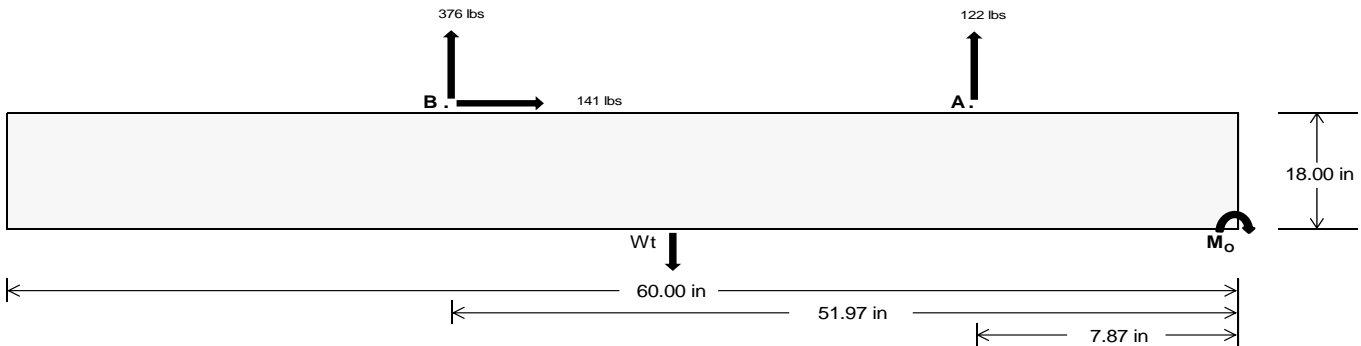
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>511.37</u>	<u>1566.57</u>	k
Compressive Load =	<u>2277.82</u>	<u>1670.25</u>	k
Lateral Load =	<u>4.10</u>	<u>588.58</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23021.8$ in-lbs
Resisting Force Required = 767.39 lbs
S.F. = 1.67
Weight Required = 1278.99 lbs
Minimum Width = 23 in
Weight Provided = 2084.38 lbs

Sliding

Force = 141.44 lbs
Friction = 0.4
Weight Required = 353.59 lbs
Resisting Weight = 2084.38 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 141.44 lbs
Cohesion = 130 psf
Area = 9.58 ft²
Resisting = 1042.19 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 23in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 23in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 23in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.92 \text{ ft}) =$

Ballast Width			
23 in	24 in	25 in	26 in
2084 lbs	2175 lbs	2266 lbs	2356 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in
F_A	860 lbs	860 lbs	860 lbs	860 lbs	648 lbs	648 lbs	648 lbs	648 lbs	1072 lbs	1072 lbs	1072 lbs	1072 lbs	-243 lbs	-243 lbs	-243 lbs	-243 lbs
F_B	632 lbs	632 lbs	632 lbs	632 lbs	474 lbs	474 lbs	474 lbs	474 lbs	785 lbs	785 lbs	785 lbs	785 lbs	-751 lbs	-751 lbs	-751 lbs	-751 lbs
F_V	60 lbs	60 lbs	60 lbs	60 lbs	252 lbs	252 lbs	252 lbs	252 lbs	231 lbs	231 lbs	231 lbs	231 lbs	-283 lbs	-283 lbs	-283 lbs	-283 lbs
P_{total}	3576 lbs	3667 lbs	3758 lbs	3848 lbs	3206 lbs	3297 lbs	3387 lbs	3478 lbs	3942 lbs	4032 lbs	4123 lbs	4213 lbs	256 lbs	311 lbs	366 lbs	419 lbs
M	517 lbs-ft	517 lbs-ft	517 lbs-ft	517 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft	706 lbs-ft	885 lbs-ft	885 lbs-ft	885 lbs-ft	885 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft
e	0.14 ft	0.14 ft	0.14 ft	0.13 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.22 ft	0.22 ft	0.21 ft	0.21 ft	1.96 ft	1.62 ft	1.38 ft	1.20 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	308.4 psf	304.7 psf	301.2 psf	298.0 psf	246.1 psf	244.9 psf	243.8 psf	242.8 psf	300.4 psf	297.0 psf	293.8 psf	290.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	437.9 psf	428.7 psf	420.3 psf	412.5 psf	423.0 psf	414.4 psf	406.6 psf	399.3 psf	522.1 psf	509.4 psf	497.8 psf	487.0 psf	165.4 psf	117.4 psf	104.0 psf	99.2 psf

Maximum Bearing Pressure = 522 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

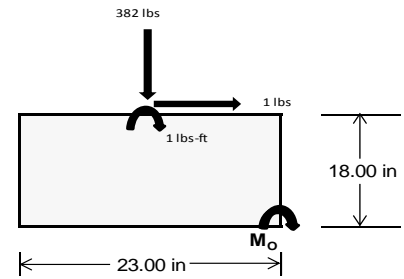
Overturning Check

$M_o = 364.0 \text{ ft-lbs}$
 Resisting Force Required = 379.82 lbs
 S.F. = 1.67
 Weight Required = 633.04 lbs
 Minimum Width = 23 in
 Weight Provided = 2084.38 lbs

A minimum 60in long x 23in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	23 in			23 in			23 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	91 lbs	255 lbs	86 lbs	387 lbs	1207 lbs	382 lbs	27 lbs	75 lbs	25 lbs
F_v	3 lbs	3 lbs	0 lbs	17 lbs	16 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2671 lbs	2836 lbs	2666 lbs	2844 lbs	3664 lbs	2839 lbs	781 lbs	829 lbs	780 lbs
M	5 lbs-ft	5 lbs-ft	0 lbs-ft	29 lbs-ft	24 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.32 ft	1.91 ft	1.92 ft	1.90 ft	1.90 ft	1.92 ft	1.91 ft	1.91 ft	1.92 ft
f_{min}	277.1 sqft	294.4 sqft	278.1 sqft	287.2 sqft	374.5 sqft	295.5 sqft	81.0 sqft	86.1 sqft	81.3 sqft
f_{max}	280.3 psf	297.4 psf	278.3 psf	306.2 psf	390.1 psf	296.9 psf	82.0 psf	87.0 psf	81.4 psf



Maximum Bearing Pressure = 390 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 23in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

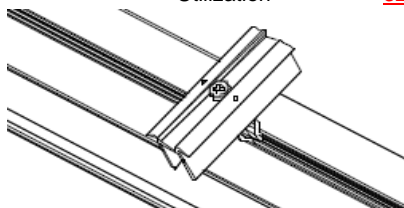
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

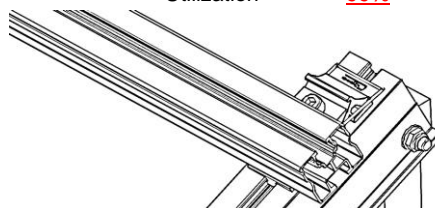
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.384 k
Allowable Uplift =	1.214 k
Utilization =	<u>32%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.106 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

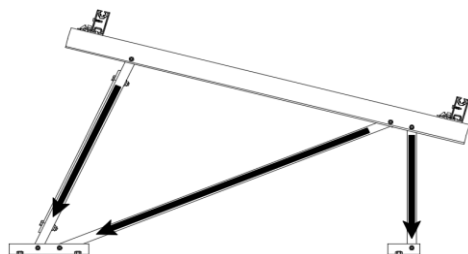
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.752 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>31%</u>

Diagonal Strut

Maximum Axial Load =	0.164 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.408 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

Bracing

Maximum Axial Load =	0.028 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.047 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 99.00 \text{ in}$$

$$J = 0.427$$

$$206.479$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 99.00 \text{ in}$$

$$J = 0.427$$

$$224.369$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.7 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.782 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	102.071	2	358.384	1	.034	2	0	1	0	1	0	1
2		min	-141.762	3	-366.172	3	-.106	3	0	3	0	1	0	1
3	N7	max	0	15	587.369	1	-.05	15	0	15	0	1	0	1
4		min	-.192	1	-112.158	3	-1.392	1	-.003	1	0	1	0	1
5	N15	max	0	15	1752.17	1	.502	1	.001	1	0	1	0	1
6		min	-2.036	1	-393.358	3	-.227	3	0	3	0	1	0	1
7	N16	max	427.481	2	1284.805	1	-.185	10	0	1	0	1	0	1
8		min	-452.751	3	-1205.056	3	-26.187	1	0	3	0	1	0	1
9	N23	max	0	15	587.27	1	3.15	1	.006	1	0	1	0	1
10		min	-.192	1	-111.753	3	.106	15	0	15	0	1	0	1
11	N24	max	102.478	2	363.855	1	25.737	3	.002	1	0	1	0	1
12		min	-141.817	3	-363.193	3	.038	10	0	3	0	1	0	1
13	Totals:	max	630.068	2	4933.853	1	0	3						
14		min	-736.694	3	-2551.69	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	428.124	1	.66	4	.863	1	0	15	0	3	0	1
2			min	-368.07	3	.157	15	-.054	3	-.001	1	0	2	0	1
3		2	max	428.221	1	.622	4	.863	1	0	15	0	1	0	15
4			min	-367.998	3	.148	15	-.054	3	-.001	1	0	10	0	4
5		3	max	428.317	1	.584	4	.863	1	0	15	0	1	0	15
6			min	-367.925	3	.139	15	-.054	3	-.001	1	0	15	0	4
7		4	max	428.414	1	.547	4	.863	1	0	15	0	1	0	15
8			min	-367.853	3	.13	15	-.054	3	-.001	1	0	12	0	4
9		5	max	428.51	1	.509	4	.863	1	0	15	0	1	0	15
10			min	-367.781	3	.121	15	-.054	3	-.001	1	0	3	0	4
11		6	max	428.606	1	.471	4	.863	1	0	15	0	1	0	15
12			min	-367.708	3	.112	15	-.054	3	-.001	1	0	3	0	4
13		7	max	428.703	1	.433	4	.863	1	0	15	0	1	0	15
14			min	-367.636	3	.104	15	-.054	3	-.001	1	0	3	0	4
15		8	max	428.799	1	.395	4	.863	1	0	15	0	1	0	15
16			min	-367.564	3	.095	15	-.054	3	-.001	1	0	3	0	4
17		9	max	428.895	1	.357	4	.863	1	0	15	0	1	0	15
18			min	-367.492	3	.086	15	-.054	3	-.001	1	0	3	0	4
19		10	max	428.992	1	.32	4	.863	1	0	15	.001	1	0	15
20			min	-367.419	3	.077	15	-.054	3	-.001	1	0	3	0	4
21		11	max	429.088	1	.282	4	.863	1	0	15	.001	1	0	15
22			min	-367.347	3	.068	15	-.054	3	-.001	1	0	3	0	4
23		12	max	429.184	1	.244	4	.863	1	0	15	.001	1	0	15
24			min	-367.275	3	.059	15	-.054	3	-.001	1	0	3	0	4
25		13	max	429.281	1	.206	4	.863	1	0	15	.001	1	0	15
26			min	-367.202	3	.05	15	-.054	3	-.001	1	0	3	0	4
27		14	max	429.377	1	.168	4	.863	1	0	15	.002	1	0	15
28			min	-367.13	3	.041	15	-.054	3	-.001	1	0	3	0	4
29		15	max	429.474	1	.13	4	.863	1	0	15	.002	1	0	15
30			min	-367.058	3	.032	15	-.054	3	-.001	1	0	3	0	4
31		16	max	429.57	1	.093	4	.863	1	0	15	.002	1	0	15
32			min	-366.986	3	.024	15	-.054	3	-.001	1	0	3	0	4
33		17	max	429.666	1	.055	4	.863	1	0	15	.002	1	0	15
34			min	-366.913	3	0	1	-.054	3	-.001	1	0	3	0	4
35		18	max	429.763	1	.029	10	.863	1	0	15	.002	1	0	15
36			min	-366.841	3	-.03	1	-.054	3	-.001	1	0	3	0	4
37		19	max	429.859	1	.005	10	.863	1	0	15	.002	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-366.769	3	-.059	1	-.054	3	-.001	1	0	3	0	4
39	M3	1	max	36.749	10	1.811	4	-.023	15	0	.002	1	0	4
40		min	-123.867	1	.427	15	-.738	1	0	1	0	15	0	15
41		2	max	36.693	10	1.633	4	-.023	15	0	.002	1	0	4
42		min	-123.934	1	.385	15	-.738	1	0	1	0	15	0	15
43		3	max	36.638	10	1.455	4	-.023	15	0	.002	1	0	10
44		min	-124.001	1	.343	15	-.738	1	0	1	0	15	0	1
45		4	max	36.582	10	1.277	4	-.023	15	0	.002	1	0	15
46		min	-124.068	1	.301	15	-.738	1	0	1	0	15	0	1
47		5	max	36.526	10	1.099	4	-.023	15	0	.001	1	0	15
48		min	-124.135	1	.26	15	-.738	1	0	1	0	15	0	4
49		6	max	36.47	10	.921	4	-.023	15	0	.001	1	0	15
50		min	-124.202	1	.218	15	-.738	1	0	1	0	15	0	4
51		7	max	36.414	10	.743	4	-.023	15	0	.001	1	0	15
52		min	-124.269	1	.176	15	-.738	1	0	1	0	15	0	4
53		8	max	36.358	10	.565	4	-.023	15	0	.001	1	0	15
54		min	-124.336	1	.134	15	-.738	1	0	1	0	15	0	4
55		9	max	36.302	10	.387	4	-.023	15	0	0	1	0	15
56		min	-124.404	1	.092	15	-.738	1	0	1	0	15	-.001	4
57		10	max	36.246	10	.209	4	-.023	15	0	0	1	0	15
58		min	-124.471	1	.05	15	-.738	1	0	1	0	15	-.001	4
59		11	max	36.19	10	.032	10	-.023	15	0	0	1	0	15
60		min	-124.538	1	-.004	1	-.738	1	0	1	0	15	-.001	4
61		12	max	36.134	10	-.033	15	-.023	15	0	0	1	0	15
62		min	-124.605	1	-.147	4	-.738	1	0	1	0	12	-.001	4
63		13	max	36.078	10	-.075	15	-.023	15	0	0	1	0	15
64		min	-124.672	1	-.325	4	-.738	1	0	1	0	12	-.001	4
65		14	max	36.022	10	-.117	15	-.023	15	0	0	1	0	15
66		min	-124.739	1	-.503	4	-.738	1	0	1	0	3	-.001	4
67		15	max	35.967	10	-.159	15	-.023	15	0	0	15	0	15
68		min	-124.806	1	-.681	4	-.738	1	0	1	0	1	0	4
69		16	max	35.911	10	-.201	15	-.023	15	0	0	15	0	15
70		min	-124.873	1	-.859	4	-.738	1	0	1	0	1	0	4
71		17	max	35.855	10	-.243	15	-.023	15	0	0	15	0	15
72		min	-124.94	1	-1.037	4	-.738	1	0	1	0	1	0	4
73		18	max	35.799	10	-.284	15	-.023	15	0	0	15	0	15
74		min	-125.007	1	-1.215	4	-.738	1	0	1	0	1	0	4
75		19	max	35.743	10	-.326	15	-.023	15	0	0	15	0	1
76		min	-125.074	1	-1.393	4	-.738	1	0	1	0	1	0	1
77	M4	1	max	586.205	1	0	1	-.05	15	0	0	3	0	1
78		min	-113.031	3	0	1	-1.544	1	0	1	0	1	0	1
79		2	max	586.269	1	0	1	-.05	15	0	0	12	0	1
80		min	-112.983	3	0	1	-1.544	1	0	1	0	1	0	1
81		3	max	586.334	1	0	1	-.05	15	0	0	15	0	1
82		min	-112.934	3	0	1	-1.544	1	0	1	0	1	0	1
83		4	max	586.399	1	0	1	-.05	15	0	0	15	0	1
84		min	-112.886	3	0	1	-1.544	1	0	1	0	1	0	1
85		5	max	586.463	1	0	1	-.05	15	0	0	15	0	1
86		min	-112.837	3	0	1	-1.544	1	0	1	0	1	0	1
87		6	max	586.528	1	0	1	-.05	15	0	0	15	0	1
88		min	-112.789	3	0	1	-1.544	1	0	1	0	1	0	1
89		7	max	586.593	1	0	1	-.05	15	0	0	15	0	1
90		min	-112.74	3	0	1	-1.544	1	0	1	0	1	0	1
91		8	max	586.658	1	0	1	-.05	15	0	0	15	0	1
92		min	-112.692	3	0	1	-1.544	1	0	1	-.001	1	0	1
93		9	max	586.722	1	0	1	-.05	15	0	0	15	0	1
94		min	-112.643	3	0	1	-1.544	1	0	1	-.001	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	586.787	1	0	1	-.05	15	0	1	0	15	0	1
96		min	-112.594	3	0	1	-1.544	1	0	1	-.001	1	0	1
97	11	max	586.852	1	0	1	-.05	15	0	1	0	15	0	1
98		min	-112.546	3	0	1	-1.544	1	0	1	-.001	1	0	1
99	12	max	586.916	1	0	1	-.05	15	0	1	0	15	0	1
100		min	-112.497	3	0	1	-1.544	1	0	1	-.002	1	0	1
101	13	max	586.981	1	0	1	-.05	15	0	1	0	15	0	1
102		min	-112.449	3	0	1	-1.544	1	0	1	-.002	1	0	1
103	14	max	587.046	1	0	1	-.05	15	0	1	0	15	0	1
104		min	-112.4	3	0	1	-1.544	1	0	1	-.002	1	0	1
105	15	max	587.11	1	0	1	-.05	15	0	1	0	15	0	1
106		min	-112.352	3	0	1	-1.544	1	0	1	-.002	1	0	1
107	16	max	587.175	1	0	1	-.05	15	0	1	0	15	0	1
108		min	-112.303	3	0	1	-1.544	1	0	1	-.002	1	0	1
109	17	max	587.24	1	0	1	-.05	15	0	1	0	15	0	1
110		min	-112.255	3	0	1	-1.544	1	0	1	-.002	1	0	1
111	18	max	587.305	1	0	1	-.05	15	0	1	0	15	0	1
112		min	-112.206	3	0	1	-1.544	1	0	1	-.002	1	0	1
113	19	max	587.369	1	0	1	-.05	15	0	1	0	15	0	1
114		min	-112.158	3	0	1	-1.544	1	0	1	-.003	1	0	1
115	M6	1	max	1405.827	1	.642	4	.337	1	0	1	0	3	0
116		min	-1207.953	3	.155	15	-.12	3	0	15	0	1	0	1
117	2	max	1405.923	1	.605	4	.337	1	0	1	0	3	0	15
118		min	-1207.881	3	.146	15	-.12	3	0	15	0	2	0	4
119	3	max	1406.02	1	.567	4	.337	1	0	1	0	1	0	15
120		min	-1207.809	3	.137	15	-.12	3	0	15	0	15	0	4
121	4	max	1406.116	1	.529	4	.337	1	0	1	0	1	0	15
122		min	-1207.737	3	.128	15	-.12	3	0	15	0	3	0	4
123	5	max	1406.213	1	.491	4	.337	1	0	1	0	1	0	15
124		min	-1207.664	3	.119	15	-.12	3	0	15	0	3	0	4
125	6	max	1406.309	1	.453	4	.337	1	0	1	0	1	0	15
126		min	-1207.592	3	.11	15	-.12	3	0	15	0	3	0	4
127	7	max	1406.405	1	.415	4	.337	1	0	1	0	1	0	15
128		min	-1207.52	3	.101	15	-.12	3	0	15	0	3	0	4
129	8	max	1406.502	1	.378	4	.337	1	0	1	0	1	0	15
130		min	-1207.447	3	.092	15	-.12	3	0	15	0	3	0	4
131	9	max	1406.598	1	.34	4	.337	1	0	1	0	1	0	15
132		min	-1207.375	3	.083	15	-.12	3	0	15	0	3	0	4
133	10	max	1406.694	1	.302	4	.337	1	0	1	0	1	0	15
134		min	-1207.303	3	.075	15	-.12	3	0	15	0	3	0	4
135	11	max	1406.791	1	.264	4	.337	1	0	1	0	1	0	15
136		min	-1207.231	3	.066	15	-.12	3	0	15	0	3	0	4
137	12	max	1406.887	1	.226	4	.337	1	0	1	0	1	0	15
138		min	-1207.158	3	.057	15	-.12	3	0	15	0	3	0	4
139	13	max	1406.983	1	.188	4	.337	1	0	1	0	1	0	15
140		min	-1207.086	3	.048	15	-.12	3	0	15	0	3	0	4
141	14	max	1407.08	1	.154	2	.337	1	0	1	0	1	0	15
142		min	-1207.014	3	.032	9	-.12	3	0	15	0	3	0	4
143	15	max	1407.176	1	.124	2	.337	1	0	1	0	1	0	15
144		min	-1206.941	3	.008	9	-.12	3	0	15	0	3	0	4
145	16	max	1407.273	1	.097	10	.337	1	0	1	0	1	0	15
146		min	-1206.869	3	-.019	1	-.12	3	0	15	0	3	0	4
147	17	max	1407.369	1	.072	10	.337	1	0	1	0	1	0	15
148		min	-1206.797	3	-.048	1	-.12	3	0	15	0	3	0	4
149	18	max	1407.465	1	.048	10	.337	1	0	1	0	1	0	15
150		min	-1206.725	3	-.078	1	-.12	3	0	15	0	3	0	4
151	19	max	1407.562	1	.023	10	.337	1	0	1	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1206.652	3	-.107	1	-.12	3	0	15	0	3	0	4
153	M7	1	max	164.064	2	1.803	4	.015	1	0	2	0	3	4
154		min	-172.001	9	.426	15	-.008	3	0	3	0	3	0	15
155		2	max	163.997	2	1.625	4	.015	1	0	2	0	2	2
156		min	-172.057	9	.384	15	-.008	3	0	3	0	3	0	15
157		3	max	163.93	2	1.447	4	.015	1	0	2	0	2	2
158		min	-172.113	9	.342	15	-.008	3	0	3	0	3	0	9
159		4	max	163.862	2	1.269	4	.015	1	0	2	0	2	10
160		min	-172.169	9	.3	15	-.008	3	0	3	0	3	0	1
161		5	max	163.795	2	1.091	4	.015	1	0	2	0	2	15
162		min	-172.225	9	.259	15	-.008	3	0	3	0	3	0	1
163		6	max	163.728	2	.913	4	.015	1	0	2	0	2	15
164		min	-172.281	9	.217	15	-.008	3	0	3	0	3	0	4
165		7	max	163.661	2	.735	4	.015	1	0	2	0	2	15
166		min	-172.337	9	.175	15	-.008	3	0	3	0	3	0	4
167		8	max	163.594	2	.557	4	.015	1	0	2	0	2	15
168		min	-172.393	9	.133	15	-.008	3	0	3	0	3	0	4
169		9	max	163.527	2	.379	4	.015	1	0	2	0	2	15
170		min	-172.449	9	.091	15	-.008	3	0	3	0	3	-.001	4
171		10	max	163.46	2	.201	4	.015	1	0	2	0	2	15
172		min	-172.505	9	.049	15	-.008	3	0	3	0	3	-.001	4
173		11	max	163.393	2	.053	2	.015	1	0	2	0	2	15
174		min	-172.561	9	-.022	9	-.008	3	0	3	0	3	-.001	4
175		12	max	163.326	2	-.034	15	.015	1	0	2	0	2	15
176		min	-172.616	9	-.157	1	-.008	3	0	3	0	3	-.001	4
177		13	max	163.259	2	-.076	15	.015	1	0	2	0	2	15
178		min	-172.672	9	-.333	4	-.008	3	0	3	0	3	-.001	4
179		14	max	163.192	2	-.118	15	.015	1	0	2	0	2	15
180		min	-172.728	9	-.511	4	-.008	3	0	3	0	3	-.001	4
181		15	max	163.124	2	-.16	15	.015	1	0	2	0	2	15
182		min	-172.784	9	-.689	4	-.008	3	0	3	0	3	0	4
183		16	max	163.057	2	-.202	15	.015	1	0	2	0	2	15
184		min	-172.84	9	-.867	4	-.008	3	0	3	0	3	0	4
185		17	max	162.99	2	-.244	15	.015	1	0	2	0	2	15
186		min	-172.896	9	-1.045	4	-.008	3	0	3	0	3	0	4
187		18	max	162.923	2	-.285	15	.015	1	0	2	0	2	15
188		min	-172.952	9	-1.223	4	-.008	3	0	3	0	3	0	4
189		19	max	162.856	2	-.327	15	.015	1	0	2	0	2	1
190		min	-173.008	9	-1.401	4	-.008	3	0	3	0	3	0	1
191	M8	1	max	1751.006	1	0	1	.703	1	0	1	0	15	0
192		min	-394.232	3	0	1	-.213	3	0	1	0	1	0	1
193		2	max	1751.07	1	0	1	.703	1	0	1	0	1	0
194		min	-394.183	3	0	1	-.213	3	0	1	0	3	0	1
195		3	max	1751.135	1	0	1	.703	1	0	1	0	1	0
196		min	-394.135	3	0	1	-.213	3	0	1	0	3	0	1
197		4	max	1751.2	1	0	1	.703	1	0	1	0	1	0
198		min	-394.086	3	0	1	-.213	3	0	1	0	3	0	1
199		5	max	1751.265	1	0	1	.703	1	0	1	0	1	0
200		min	-394.037	3	0	1	-.213	3	0	1	0	3	0	1
201		6	max	1751.329	1	0	1	.703	1	0	1	0	1	0
202		min	-393.989	3	0	1	-.213	3	0	1	0	3	0	1
203		7	max	1751.394	1	0	1	.703	1	0	1	0	1	0
204		min	-393.94	3	0	1	-.213	3	0	1	0	3	0	1
205		8	max	1751.459	1	0	1	.703	1	0	1	0	1	0
206		min	-393.892	3	0	1	-.213	3	0	1	0	3	0	1
207		9	max	1751.523	1	0	1	.703	1	0	1	0	1	0
208		min	-393.843	3	0	1	-.213	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1751.588	1	0	1	.703	1	0	1	0	1	0	1
210			min	-393.795	3	0	1	-.213	3	0	1	0	3	0	1
211		11	max	1751.653	1	0	1	.703	1	0	1	0	1	0	1
212			min	-393.746	3	0	1	-.213	3	0	1	0	3	0	1
213		12	max	1751.718	1	0	1	.703	1	0	1	0	1	0	1
214			min	-393.698	3	0	1	-.213	3	0	1	0	3	0	1
215		13	max	1751.782	1	0	1	.703	1	0	1	0	1	0	1
216			min	-393.649	3	0	1	-.213	3	0	1	0	3	0	1
217		14	max	1751.847	1	0	1	.703	1	0	1	0	1	0	1
218			min	-393.601	3	0	1	-.213	3	0	1	0	3	0	1
219		15	max	1751.912	1	0	1	.703	1	0	1	0	1	0	1
220			min	-393.552	3	0	1	-.213	3	0	1	0	3	0	1
221		16	max	1751.976	1	0	1	.703	1	0	1	0	1	0	1
222			min	-393.504	3	0	1	-.213	3	0	1	0	3	0	1
223		17	max	1752.041	1	0	1	.703	1	0	1	.001	1	0	1
224			min	-393.455	3	0	1	-.213	3	0	1	0	3	0	1
225		18	max	1752.106	1	0	1	.703	1	0	1	.001	1	0	1
226			min	-393.407	3	0	1	-.213	3	0	1	0	3	0	1
227		19	max	1752.17	1	0	1	.703	1	0	1	.001	1	0	1
228			min	-393.358	3	0	1	-.213	3	0	1	0	3	0	1
229	M10	1	max	438.328	1	.647	4	-.004	15	.001	1	0	2	0	1
230			min	-358.622	3	.155	15	-.118	1	0	3	0	3	0	1
231		2	max	438.424	1	.61	4	-.004	15	.001	1	0	2	0	15
232			min	-358.55	3	.146	15	-.118	1	0	3	0	3	0	4
233		3	max	438.52	1	.572	4	-.004	15	.001	1	0	2	0	15
234			min	-358.477	3	.137	15	-.118	1	0	3	0	3	0	4
235		4	max	438.617	1	.534	4	-.004	15	.001	1	0	2	0	15
236			min	-358.405	3	.128	15	-.118	1	0	3	0	3	0	4
237		5	max	438.713	1	.496	4	-.004	15	.001	1	0	2	0	15
238			min	-358.333	3	.12	15	-.118	1	0	3	0	3	0	4
239		6	max	438.81	1	.458	4	-.004	15	.001	1	0	2	0	15
240			min	-358.261	3	.111	15	-.118	1	0	3	0	1	0	4
241		7	max	438.906	1	.42	4	-.004	15	.001	1	0	15	0	15
242			min	-358.188	3	.102	15	-.118	1	0	3	0	1	0	4
243		8	max	439.002	1	.383	4	-.004	15	.001	1	0	15	0	15
244			min	-358.116	3	.093	15	-.118	1	0	3	0	1	0	4
245		9	max	439.099	1	.345	4	-.004	15	.001	1	0	15	0	15
246			min	-358.044	3	.084	15	-.118	1	0	3	0	1	0	4
247		10	max	439.195	1	.307	4	-.004	15	.001	1	0	15	0	15
248			min	-357.972	3	.075	15	-.118	1	0	3	0	1	0	4
249		11	max	439.291	1	.269	4	-.004	15	.001	1	0	15	0	15
250			min	-357.899	3	.066	15	-.118	1	0	3	0	1	0	4
251		12	max	439.388	1	.231	4	-.004	15	.001	1	0	15	0	15
252			min	-357.827	3	.057	15	-.118	1	0	3	0	1	0	4
253		13	max	439.484	1	.193	4	-.004	15	.001	1	0	15	0	15
254			min	-357.755	3	.048	15	-.118	1	0	3	0	1	0	4
255		14	max	439.58	1	.156	4	-.004	15	.001	1	0	15	0	15
256			min	-357.682	3	.025	1	-.118	1	0	3	0	1	0	4
257		15	max	439.677	1	.119	3	-.004	15	.001	1	0	15	0	15
258			min	-357.61	3	-.004	1	-.118	1	0	3	0	1	0	4
259		16	max	439.773	1	.097	3	-.004	15	.001	1	0	15	0	15
260			min	-357.538	3	-.034	1	-.118	1	0	3	0	1	0	4
261		17	max	439.87	1	.075	3	-.004	15	.001	1	0	15	0	15
262			min	-357.466	3	-.063	1	-.118	1	0	3	0	1	0	4
263		18	max	439.966	1	.053	3	-.004	15	.001	1	0	15	0	15
264			min	-357.393	3	-.093	1	-.118	1	0	3	0	1	0	4
265		19	max	440.062	1	.031	3	-.004	15	.001	1	0	15	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-357.321	3	-.122	1	-.118	1	0	3	0	1	0	4
267		1	max	36.185	10	1.816	4	.874	1	.001	1	0	3	0	4
268			min	-123.676	1	.428	15	.02	12	0	15	-.002	1	0	15
269		2	max	36.13	10	1.638	4	.874	1	.001	1	0	3	0	4
270			min	-123.743	1	.386	15	.02	12	0	15	-.002	1	0	15
271		3	max	36.074	10	1.46	4	.874	1	.001	1	0	3	0	2
272			min	-123.81	1	.344	15	.02	12	0	15	-.002	1	0	3
273		4	max	36.018	10	1.282	4	.874	1	.001	1	0	3	0	15
274			min	-123.877	1	.302	15	.02	12	0	15	-.002	1	0	4
275		5	max	35.962	10	1.104	4	.874	1	.001	1	0	3	0	15
276			min	-123.944	1	.26	15	.02	12	0	15	-.001	1	0	4
277		6	max	35.906	10	.926	4	.874	1	.001	1	0	3	0	15
278			min	-124.012	1	.218	15	.02	12	0	15	-.001	1	0	4
279		7	max	35.85	10	.748	4	.874	1	.001	1	0	3	0	15
280			min	-124.079	1	.176	15	.02	12	0	15	0	1	0	4
281		8	max	35.794	10	.57	4	.874	1	.001	1	0	3	0	15
282			min	-124.146	1	.135	15	.02	12	0	15	0	1	0	4
283	9	max	35.738	10	.392	4	.874	1	.001	1	0	3	0	15	
284		min	-124.213	1	.093	15	.02	12	0	15	0	1	-.001	4	
285	10	max	35.682	10	.214	4	.874	1	.001	1	0	3	0	15	
286		min	-124.28	1	.051	15	.02	12	0	15	0	1	-.001	4	
287	11	max	35.626	10	.051	2	.874	1	.001	1	0	3	0	15	
288		min	-124.347	1	.002	3	.02	12	0	15	0	1	-.001	4	
289	12	max	35.57	10	-.033	15	.874	1	.001	1	0	3	0	15	
290		min	-124.414	1	-.142	4	.02	12	0	15	0	2	-.001	4	
291	13	max	35.514	10	-.075	15	.874	1	.001	1	0	3	0	15	
292		min	-124.481	1	-.32	4	.02	12	0	15	0	2	-.001	4	
293	14	max	35.459	10	-.116	15	.874	1	.001	1	0	1	0	15	
294		min	-124.548	1	-.498	4	.02	12	0	15	0	15	-.001	4	
295	15	max	35.403	10	-.158	15	.874	1	.001	1	0	1	0	15	
296		min	-124.615	1	-.676	4	.02	12	0	15	0	15	0	4	
297	16	max	35.347	10	-.2	15	.874	1	.001	1	0	1	0	15	
298		min	-124.682	1	-.854	4	.02	12	0	15	0	15	0	4	
299	17	max	35.291	10	-.242	15	.874	1	.001	1	0	1	0	15	
300		min	-124.75	1	-1.032	4	.02	12	0	15	0	15	0	4	
301	18	max	35.235	10	-.284	15	.874	1	.001	1	.001	1	0	15	
302		min	-124.817	1	-1.21	4	.02	12	0	15	0	15	0	4	
303	19	max	35.179	10	-.326	15	.874	1	.001	1	.001	1	0	1	
304		min	-124.884	1	-1.388	4	.02	12	0	15	0	15	0	1	
305	M12	1	max	586.105	1	0	1	3.487	1	0	1	0	1	0	1
306		min	-112.627	3	0	1	.106	15	0	1	0	3	0	1	
307	2	max	586.17	1	0	1	3.487	1	0	1	0	1	0	1	
308		min	-112.578	3	0	1	.106	15	0	1	0	12	0	1	
309	3	max	586.235	1	0	1	3.487	1	0	1	0	1	0	1	
310		min	-112.53	3	0	1	.106	15	0	1	0	15	0	1	
311	4	max	586.299	1	0	1	3.487	1	0	1	0	1	0	1	
312		min	-112.481	3	0	1	.106	15	0	1	0	15	0	1	
313	5	max	586.364	1	0	1	3.487	1	0	1	.001	1	0	1	
314		min	-112.433	3	0	1	.106	15	0	1	0	15	0	1	
315	6	max	586.429	1	0	1	3.487	1	0	1	.002	1	0	1	
316		min	-112.384	3	0	1	.106	15	0	1	0	15	0	1	
317	7	max	586.494	1	0	1	3.487	1	0	1	.002	1	0	1	
318		min	-112.336	3	0	1	.106	15	0	1	0	15	0	1	
319	8	max	586.558	1	0	1	3.487	1	0	1	.002	1	0	1	
320		min	-112.287	3	0	1	.106	15	0	1	0	15	0	1	
321	9	max	586.623	1	0	1	3.487	1	0	1	.003	1	0	1	
322		min	-112.239	3	0	1	.106	15	0	1	0	15	0	1	





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380			min	-114.331	1	-161.695	3	-70.297	1	0	1	-.134	1	-.007	3
381	M5	1	max	251.888	1	1140.143	3	-.068	10	0	1	.004	1	.02	3
382			min	6.802	12	-1413.652	1	-22.921	3	0	3	0	10	-.03	1
383		2	max	251.96	1	1139.94	3	-.068	10	0	1	0	2	.277	1
384			min	6.838	12	-1413.922	1	-22.921	3	0	3	-.002	3	-.227	3
385		3	max	301.193	1	10.416	9	2.628	3	0	3	0	2	.578	1
386			min	-32.025	3	-77.11	3	-.339	2	0	1	-.007	3	-.469	3
387		4	max	301.265	1	10.191	9	2.628	3	0	3	0	2	.581	1
388			min	-31.97	3	-77.312	3	-.339	2	0	1	-.007	3	-.452	3
389		5	max	301.337	1	9.966	9	2.628	3	0	3	0	2	.584	1
390			min	-31.916	3	-77.514	3	-.339	2	0	1	-.006	3	-.436	3
391		6	max	301.409	1	9.742	9	2.628	3	0	3	0	2	.588	1
392			min	-31.862	3	-77.716	3	-.339	2	0	1	-.005	3	-.419	3
393		7	max	301.482	1	9.517	9	2.628	3	0	3	0	2	.591	1
394			min	-31.808	3	-77.919	3	-.339	2	0	1	-.005	3	-.402	3
395		8	max	301.554	1	9.292	9	2.628	3	0	3	0	2	.595	1
396			min	-31.754	3	-78.121	3	-.339	2	0	1	-.004	3	-.385	3
397		9	max	301.626	1	9.067	9	2.628	3	0	3	0	10	.598	1
398			min	-31.699	3	-78.323	3	-.339	2	0	1	-.004	3	-.368	3
399		10	max	301.699	1	8.843	9	2.628	3	0	3	0	10	.602	1
400			min	-31.645	3	-78.526	3	-.339	2	0	1	-.003	3	-.351	3
401		11	max	301.771	1	8.618	9	2.628	3	0	3	0	10	.605	1
402			min	-31.591	3	-78.728	3	-.339	2	0	1	-.003	1	-.334	3
403		12	max	301.843	1	8.393	9	2.628	3	0	3	0	10	.609	1
404			min	-31.537	3	-78.93	3	-.339	2	0	1	-.002	1	-.317	3
405		13	max	301.915	1	8.168	9	2.628	3	0	3	0	10	.613	1
406			min	-31.483	3	-79.132	3	-.339	2	0	1	-.002	1	-.3	3
407		14	max	301.988	1	7.944	9	2.628	3	0	3	0	10	.616	1
408			min	-31.428	3	-79.335	3	-.339	2	0	1	-.002	1	-.283	3
409		15	max	302.06	1	7.719	9	2.628	3	0	3	0	15	.62	1
410			min	-31.374	3	-79.537	3	-.339	2	0	1	-.001	1	-.265	3
411		16	max	256.161	2	54.25	10	2.607	3	0	1	0	3	.625	1
412			min	-112.685	3	-150.197	3	-.338	2	0	15	-.001	1	-.247	3
413		17	max	256.233	2	54.026	10	2.607	3	0	1	0	3	.647	1
414			min	-112.631	3	-150.4	3	-.338	2	0	15	0	1	-.215	3
415		18	max	-7.244	12	1580.313	1	2.392	3	0	3	.001	3	.311	1
416			min	-252.583	1	-532.2	3	-.062	2	0	1	0	2	-.101	3
417		19	max	-7.208	12	1580.044	1	2.392	3	0	3	.002	3	.014	3
418			min	-252.511	1	-532.403	3	-.062	2	0	1	0	2	-.031	1
419	M9	1	max	114.267	1	344.84	3	90.376	1	0	3	-.004	15	.015	1
420			min	3.503	15	-427.389	1	2.891	15	0	1	-.135	1	-.01	3
421		2	max	114.34	1	344.637	3	90.376	1	0	3	-.002	12	.108	1
422			min	3.525	15	-427.659	1	2.891	15	0	1	-.115	1	-.085	3
423		3	max	130.41	1	6.982	9	64.15	1	0	1	.002	3	.198	1
424			min	-6.261	3	-23.378	3	.896	12	0	15	-.094	1	-.158	3
425		4	max	130.482	1	6.757	9	64.15	1	0	1	.002	3	.198	1
426			min	-6.207	3	-23.581	3	.896	12	0	15	-.08	1	-.153	3
427		5	max	130.555	1	6.532	9	64.15	1	0	1	.003	3	.198	1
428			min	-6.153	3	-23.783	3	.896	12	0	15	-.066	1	-.148	3
429		6	max	130.627	1	6.307	9	64.15	1	0	1	.003	3	.198	1
430			min	-6.099	3	-23.985	3	.896	12	0	15	-.052	1	-.143	3
431		7	max	130.699	1	6.083	9	64.15	1	0	1	.003	3	.199	1
432			min	-6.044	3	-24.188	3	.896	12	0	15	-.038	1	-.137	3
433		8	max	130.771	1	5.858	9	64.15	1	0	1	.003	3	.199	1
434			min	-5.99	3	-24.39	3	.896	12	0	15	-.025	1	-.132	3
435		9	max	130.844	1	5.633	9	64.15	1	0	1	.004	3	.199	1
436			min	-5.936	3	-24.592	3	.896	12	0	15	-.011	1	-.127	3



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	130.916	1	5.408	9	64.15	1	0	1	.004	3	.199	1
438			min	-5.882	3	-24.795	3	.896	12	0	15	0	2	-.121	3
439		11	max	130.988	1	5.184	9	64.15	1	0	1	.017	1	.2	1
440			min	-5.828	3	-24.997	3	.896	12	0	15	0	15	-.116	3
441		12	max	131.06	1	4.959	9	64.15	1	0	1	.031	1	.2	1
442			min	-5.773	3	-25.199	3	.896	12	0	15	0	15	-.111	3
443		13	max	131.133	1	4.734	9	64.15	1	0	1	.045	1	.2	1
444			min	-5.719	3	-25.401	3	.896	12	0	15	.001	15	-.105	3
445		14	max	131.205	1	4.509	9	64.15	1	0	1	.059	1	.201	1
446			min	-5.665	3	-25.604	3	.896	12	0	15	.002	15	-.1	3
447		15	max	131.277	1	4.285	9	64.15	1	0	1	.073	1	.202	1
448			min	-5.611	3	-25.806	3	.896	12	0	15	.002	15	-.094	3
449		16	max	69.303	2	9.603	10	64.969	1	0	15	.088	1	.203	1
450			min	-34.75	3	-85.314	1	.919	12	0	1	.003	15	-.088	3
451		17	max	69.376	2	9.378	10	64.969	1	0	15	.103	1	.221	1
452			min	-34.695	3	-85.584	1	.919	12	0	1	.003	15	-.077	3
453		18	max	-3.52	15	479.405	1	68.357	1	0	1	.117	1	.12	1
454			min	-114.177	1	-161.491	3	1.109	12	0	3	.004	15	-.042	3
455		19	max	-3.498	15	479.135	1	68.357	1	0	1	.132	1	.016	1
456			min	-114.105	1	-161.693	3	1.109	12	0	3	.004	15	-.007	3
457	M13	1	max	90.547	1	426.817	1	-3.503	15	.015	1	.135	1	0	1
458			min	2.891	15	-344.83	3	-114.255	1	-.01	3	.004	15	0	3
459		2	max	90.547	1	301.091	1	-2.686	15	.015	1	.042	1	.269	3
460			min	2.891	15	-243.181	3	-87.536	1	-.01	3	.001	15	-.334	1
461		3	max	90.547	1	175.365	1	-1.868	15	.015	1	.001	3	.446	3
462			min	2.891	15	-141.533	3	-60.817	1	-.01	3	-.026	1	-.552	1
463		4	max	90.547	1	49.638	1	-1.05	15	.015	1	0	12	.529	3
464			min	2.891	15	-39.884	3	-34.097	1	-.01	3	-.069	1	-.655	1
465		5	max	90.547	1	61.765	3	-.233	15	.015	1	-.002	12	.519	3
466			min	2.891	15	-76.088	1	-7.378	1	-.01	3	-.088	1	-.643	1
467		6	max	90.547	1	163.413	3	19.341	1	.015	1	-.002	12	.416	3
468			min	2.891	15	-201.814	1	.254	12	-.01	3	-.083	1	-.516	1
469		7	max	90.547	1	265.062	3	46.061	1	.015	1	-.001	12	.219	3
470			min	2.891	15	-327.54	1	1.051	12	-.01	3	-.053	1	-.273	1
471		8	max	90.547	1	366.71	3	72.78	1	.015	1	.001	1	.085	1
472			min	2.891	15	-453.267	1	1.849	12	-.01	3	0	12	-.07	3
473		9	max	90.547	1	468.359	3	99.499	1	.015	1	.08	1	.558	1
474			min	2.891	15	-578.993	1	2.647	12	-.01	3	.002	12	-.453	3
475		10	max	90.547	1	570.008	3	126.219	1	.011	2	.184	1	1.146	1
476			min	2.891	15	-704.719	1	3.445	12	-.015	1	.005	12	-.929	3
477		11	max	68.646	1	578.993	1	-2.558	12	.01	3	.077	1	.558	1
478			min	2.12	15	-468.359	3	-98.973	1	-.015	1	0	12	-.453	3
479		12	max	68.646	1	453.267	1	-1.76	12	.01	3	0	2	.085	1
480			min	2.12	15	-366.71	3	-72.253	1	-.015	1	-.002	3	-.07	3
481		13	max	68.646	1	327.54	1	-.963	12	.01	3	-.002	15	.219	3
482			min	2.12	15	-265.062	3	-45.534	1	-.015	1	-.056	1	-.273	1
483		14	max	68.646	1	201.814	1	-.165	12	.01	3	-.003	15	.416	3
484			min	2.12	15	-163.413	3	-18.815	1	-.015	1	-.085	1	-.516	1
485		15	max	68.646	1	76.088	1	7.905	1	.01	3	-.003	15	.519	3
486			min	2.12	15	-61.765	3	.25	15	-.015	1	-.09	1	-.643	1
487		16	max	68.646	1	39.884	3	34.624	1	.01	3	-.002	12	.529	3
488			min	2.12	15	-49.639	1	1.068	15	-.015	1	-.071	1	-.655	1
489		17	max	68.646	1	141.533	3	61.343	1	.01	3	0	12	.446	3
490			min	2.12	15	-175.365	1	1.886	15	-.015	1	-.027	1	-.552	1
491		18	max	68.646	1	243.181	3	88.063	1	.01	3	.042	1	.269	3
492			min	2.12	15	-301.091	1	2.703	15	-.015	1	.001	15	-.334	1
493		19	max	68.646	1	344.83	3	114.782	1	.01	3	.135	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	2.12	15	-426.818	1	3.521	15	-.015	1	.004	15	0	3
495	M16	1	max	-1.109	12	479.74	1	-3.498	15	.007	3	.132	1	0	1
496			min	-68.151	1	-161.709	3	-114.115	1	-.016	1	.004	15	0	3
497		2	max	-1.109	12	338.409	1	-2.68	15	.007	3	.04	1	.126	3
498			min	-68.151	1	-114.158	3	-87.396	1	-.016	1	.001	15	-.375	1
499		3	max	-1.109	12	197.077	1	-1.863	15	.007	3	0	12	.209	3
500			min	-68.151	1	-66.608	3	-60.676	1	-.016	1	-.028	1	-.62	1
501		4	max	-1.109	12	55.745	1	-1.045	15	.007	3	-.002	15	.249	3
502			min	-68.151	1	-19.057	3	-33.957	1	-.016	1	-.071	1	-.736	1
503		5	max	-1.109	12	28.494	3	-.227	15	.007	3	-.003	15	.244	3
504			min	-68.151	1	-85.586	1	-7.238	1	-.016	1	-.09	1	-.723	1
505		6	max	-1.109	12	76.044	3	19.482	1	.007	3	-.003	15	.196	3
506			min	-68.151	1	-226.918	1	.359	12	-.016	1	-.085	1	-.579	1
507		7	max	-1.109	12	123.595	3	46.201	1	.007	3	-.002	15	.105	3
508			min	-68.151	1	-368.25	1	1.157	12	-.016	1	-.055	1	-.307	1
509		8	max	-1.109	12	171.145	3	72.92	1	.007	3	.001	2	.096	1
510			min	-68.151	1	-509.581	1	1.955	12	-.016	1	-.001	3	-.03	3
511		9	max	-1.109	12	218.696	3	99.64	1	.007	3	.079	1	.628	1
512			min	-68.151	1	-650.913	1	2.753	12	-.016	1	.001	12	-.209	3
513		10	max	-2.167	15	-17.106	15	126.359	1	0	15	.183	1	1.289	1
514			min	-70.115	1	-792.245	1	-5.477	3	-.016	1	.005	12	-.431	3
515		11	max	-2.167	15	650.913	1	-2.86	12	.016	1	.079	1	.628	1
516			min	-70.115	1	-218.696	3	-99.413	1	-.007	3	.002	12	-.209	3
517		12	max	-2.167	15	509.581	1	-2.062	12	.016	1	0	2	.096	1
518			min	-70.115	1	-171.145	3	-72.694	1	-.007	3	0	3	-.03	3
519		13	max	-2.167	15	368.25	1	-1.264	12	.016	1	-.002	12	.105	3
520			min	-70.115	1	-123.595	3	-45.975	1	-.007	3	-.054	1	-.307	1
521		14	max	-2.167	15	226.918	1	-.467	12	.016	1	-.002	12	.196	3
522			min	-70.115	1	-76.044	3	-19.255	1	-.007	3	-.084	1	-.579	1
523		15	max	-2.167	15	85.586	1	7.464	1	.016	1	-.002	12	.244	3
524			min	-70.115	1	-28.494	3	.234	15	-.007	3	-.089	1	-.723	1
525		16	max	-2.167	15	19.057	3	34.183	1	.016	1	-.002	12	.249	3
526			min	-70.115	1	-55.745	1	1.052	15	-.007	3	-.07	1	-.736	1
527		17	max	-2.167	15	66.608	3	60.903	1	.016	1	0	12	.209	3
528			min	-70.115	1	-197.077	1	1.87	15	-.007	3	-.027	1	-.62	1
529		18	max	-2.167	15	114.158	3	87.622	1	.016	1	.042	1	.126	3
530			min	-70.115	1	-338.409	1	2.687	15	-.007	3	.001	15	-.375	1
531		19	max	-2.167	15	161.709	3	114.341	1	.016	1	.134	1	0	1
532			min	-70.115	1	-479.74	1	3.505	15	-.007	3	.004	15	0	3
533	M15	1	max	.348	2	2.87	4	.023	3	0	1	0	1	0	1
534			min	-26.423	3	0	10	-.031	1	0	3	0	3	0	1
535		2	max	.276	2	2.551	4	.023	3	0	1	0	1	0	10
536			min	-26.477	3	0	10	-.031	1	0	3	0	3	-.001	4
537		3	max	.204	2	2.232	4	.023	3	0	1	0	1	0	10
538			min	-26.531	3	0	10	-.031	1	0	3	0	3	-.002	4
539		4	max	.132	2	1.913	4	.023	3	0	1	0	1	0	10
540			min	-26.585	3	0	10	-.031	1	0	3	0	3	-.003	4
541		5	max	.06	2	1.594	4	.023	3	0	1	0	1	0	10
542			min	-26.639	3	0	10	-.031	1	0	3	0	3	-.004	4
543		6	max	0	10	1.276	4	.023	3	0	1	0	1	0	10
544			min	-26.693	3	0	10	-.031	1	0	3	0	3	-.005	4
545		7	max	0	10	.957	4	.023	3	0	1	0	3	0	10
546			min	-26.747	3	0	10	-.031	1	0	3	0	1	-.005	4
547		8	max	0	10	.638	4	.023	3	0	1	0	3	0	10
548			min	-26.801	3	0	10	-.031	1	0	3	0	1	-.006	4
549		9	max	0	10	.319	4	.023	3	0	1	0	3	0	10
550			min	-26.855	3	0	10	-.031	1	0	3	0	1	-.006	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	10	0	1	.023	3	0	1	0	3	0	10
552			min	-26.909	3	0	1	-.031	1	0	3	0	1	-.006	4
553		11	max	0	10	0	10	.023	3	0	1	0	3	0	10
554			min	-26.963	3	-.319	4	-.031	1	0	3	0	1	-.006	4
555		12	max	0	10	0	10	.023	3	0	1	0	3	0	10
556			min	-27.017	3	-.638	4	-.031	1	0	3	0	1	-.006	4
557		13	max	0	10	0	10	.023	3	0	1	0	3	0	10
558			min	-27.071	3	-.957	4	-.031	1	0	3	0	1	-.005	4
559		14	max	0	10	0	10	.023	3	0	1	0	3	0	10
560			min	-27.125	3	-1.276	4	-.031	1	0	3	0	1	-.005	4
561		15	max	0	10	0	10	.023	3	0	1	0	3	0	10
562			min	-27.179	3	-1.594	4	-.031	1	0	3	0	1	-.004	4
563		16	max	0	10	0	10	.023	3	0	1	0	3	0	10
564			min	-27.233	3	-1.913	4	-.031	1	0	3	0	1	-.003	4
565		17	max	0	10	0	10	.023	3	0	1	0	3	0	10
566			min	-27.287	3	-2.232	4	-.031	1	0	3	0	1	-.002	4
567		18	max	0	10	0	10	.023	3	0	1	0	3	0	10
568			min	-27.341	3	-2.551	4	-.031	1	0	3	0	1	-.001	4
569		19	max	0	10	0	10	.023	3	0	1	0	3	0	1
570			min	-27.395	3	-2.87	4	-.031	1	0	3	0	1	0	1
571	M16A	1	max	-.747	10	2.87	4	.019	1	0	3	0	3	0	1
572			min	-28.204	1	.675	15	-.009	3	0	1	0	1	0	1
573		2	max	-.687	10	2.551	4	.019	1	0	3	0	3	0	15
574			min	-28.132	1	.6	15	-.009	3	0	1	0	1	-.001	4
575		3	max	-.627	10	2.232	4	.019	1	0	3	0	3	0	15
576			min	-28.06	1	.525	15	-.009	3	0	1	0	1	-.002	4
577		4	max	-.567	10	1.913	4	.019	1	0	3	0	3	0	15
578			min	-27.989	1	.45	15	-.009	3	0	1	0	1	-.003	4
579		5	max	-.507	10	1.594	4	.019	1	0	3	0	3	-.001	15
580			min	-27.917	1	.375	15	-.009	3	0	1	0	1	-.004	4
581		6	max	-.447	10	1.276	4	.019	1	0	3	0	3	-.001	15
582			min	-27.845	1	.3	15	-.009	3	0	1	0	1	-.005	4
583		7	max	-.387	10	.957	4	.019	1	0	3	0	3	-.001	15
584			min	-27.773	1	.225	15	-.009	3	0	1	0	1	-.005	4
585		8	max	-.327	10	.638	4	.019	1	0	3	0	3	-.001	15
586			min	-27.701	1	.15	15	-.009	3	0	1	0	1	-.006	4
587		9	max	-.267	10	.319	4	.019	1	0	3	0	3	-.001	15
588			min	-27.629	1	.075	15	-.009	3	0	1	0	1	-.006	4
589		10	max	-.207	10	0	1	.019	1	0	3	0	3	-.001	15
590			min	-27.557	1	0	1	-.009	3	0	1	0	1	-.006	4
591		11	max	-.147	10	-.075	15	.019	1	0	3	0	3	-.001	15
592			min	-27.485	1	-.319	4	-.009	3	0	1	0	1	-.006	4
593		12	max	-.087	10	-.15	15	.019	1	0	3	0	3	-.001	15
594			min	-27.413	1	-.638	4	-.009	3	0	1	0	1	-.006	4
595		13	max	-.027	10	-.225	15	.019	1	0	3	0	1	-.001	15
596			min	-27.341	1	-.957	4	-.009	3	0	1	0	4	-.005	4
597		14	max	.033	10	-.3	15	.019	1	0	3	0	1	-.001	15
598			min	-27.269	1	-1.276	4	-.009	3	0	1	0	3	-.005	4
599		15	max	.093	10	-.375	15	.019	1	0	3	0	1	-.001	15
600			min	-27.197	1	-1.594	4	-.009	3	0	1	0	3	-.004	4
601		16	max	.153	10	-.45	15	.019	1	0	3	0	1	0	15
602			min	-27.125	1	-1.913	4	-.009	3	0	1	0	3	-.003	4
603		17	max	.213	10	-.525	15	.019	1	0	3	0	1	0	15
604			min	-27.053	1	-2.232	4	-.009	3	0	1	0	3	-.002	4
605		18	max	.273	10	-.6	15	.019	1	0	3	0	1	0	15
606			min	-26.981	1	-2.551	4	-.009	3	0	1	0	3	-.001	4
607		19	max	.333	10	-.675	15	.019	1	0	3	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-26.909	1	-2.87	4	-.009	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.013	1	-3.026e-5	15	NC	3	NC	3	
2			min	-.003	3	-.005	3	0	3	-9.817e-4	1	4821.366	2	2341.822	1	
3			2	max	.003	1	.006	2	.012	1	-2.906e-5	15	NC	3	NC	3
4				min	-.003	3	-.005	3	0	3	-9.431e-4	1	5217.292	2	2538.703	1
5			3	max	.003	1	.005	2	.011	1	-2.786e-5	15	NC	3	NC	3
6				min	-.002	3	-.004	3	0	3	-9.046e-4	1	5680.783	2	2770.336	1
7			4	max	.003	1	.005	2	.01	1	-2.666e-5	15	NC	3	NC	3
8				min	-.002	3	-.004	3	0	3	-8.66e-4	1	6226.975	2	3045.189	1
9			5	max	.002	1	.004	2	.009	1	-2.546e-5	15	NC	3	NC	3
10				min	-.002	3	-.004	3	0	3	-8.274e-4	1	6875.726	2	3374.567	1
11			6	max	.002	1	.004	2	.008	1	-2.425e-5	15	NC	1	NC	3
12				min	-.002	3	-.004	3	0	3	-7.888e-4	1	7653.504	2	3773.833	1
13			7	max	.002	1	.004	2	.007	1	-2.305e-5	15	NC	1	NC	2
14				min	-.002	3	-.004	3	0	3	-7.502e-4	1	8596.252	2	4264.301	1
15			8	max	.002	1	.003	2	.006	1	-2.185e-5	15	NC	1	NC	2
16				min	-.002	3	-.003	3	0	3	-7.116e-4	1	9753.844	2	4876.27	1
17			9	max	.002	1	.003	2	.005	1	-2.065e-5	15	NC	1	NC	2
18				min	-.002	3	-.003	3	0	3	-6.73e-4	1	NC	1	5654.029	1
19			10	max	.002	1	.002	2	.005	1	-1.945e-5	15	NC	1	NC	2
20				min	-.001	3	-.003	3	0	3	-6.344e-4	1	NC	1	6664.49	1
21		11	max	.001	1	.002	2	.004	1	-1.825e-5	15	NC	1	NC	2	
22			min	-.001	3	-.003	3	0	3	-5.958e-4	1	NC	1	8012.829	1	
23		12	max	.001	1	.002	2	.003	1	-1.705e-5	15	NC	1	NC	2	
24			min	-.001	3	-.002	3	0	3	-5.572e-4	1	NC	1	9872.56	1	
25		13	max	.001	1	.001	2	.002	1	-1.585e-5	15	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-5.187e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-1.465e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-4.801e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	-1.345e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-4.415e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.224e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-4.029e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.104e-5	15	NC	1	NC	1	
34			min	0	3	0	3	0	3	-3.643e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-9.843e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.257e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-6.922e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.871e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.305e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.255e-6	12	NC	1	NC	1	
41			2	max	0	1	0	2	0	12	1.662e-4	1	NC	1	NC	1
42				min	0	10	0	3	0	1	5.021e-6	12	NC	1	NC	1
43			3	max	0	1	0	2	0	12	2.019e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	0	1	6.144e-6	15	NC	1	NC	1
45			4	max	0	1	0	2	0	12	2.375e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	-.001	1	7.252e-6	15	NC	1	NC	1
47			5	max	0	1	0	2	0	12	2.732e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	-.001	1	8.359e-6	15	NC	1	NC	1
49			6	max	0	1	0	2	0	3	3.089e-4	1	NC	1	NC	1
50				min	0	10	-.003	3	-.001	1	9.467e-6	15	NC	1	NC	1
51		7	max	0	1	0	2	0	3	3.446e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	10	-.004	3	0	1	1.057e-5	15	NC	1	NC	1
53		8	max	0	1	.001	2	0	3	3.802e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	1.168e-5	15	NC	1	NC	1
55		9	max	0	1	.002	2	0	3	4.159e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	2	1.279e-5	15	NC	1	NC	1
57		10	max	0	1	.002	2	0	1	4.516e-4	1	NC	1	NC	1
58			min	0	10	-.006	3	0	15	1.39e-5	15	NC	1	NC	1
59		11	max	0	1	.003	2	.001	1	4.872e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	15	1.5e-5	15	NC	1	NC	1
61		12	max	0	1	.003	2	.002	1	5.229e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	15	1.611e-5	15	NC	1	NC	1
63		13	max	0	1	.004	2	.002	1	5.586e-4	1	NC	1	NC	1
64			min	0	10	-.007	3	0	15	1.722e-5	15	NC	1	NC	1
65		14	max	.001	1	.005	2	.003	1	5.942e-4	1	NC	3	NC	1
66			min	0	10	-.007	3	0	15	1.833e-5	15	9619.549	2	NC	1
67		15	max	.001	1	.006	2	.004	1	6.299e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	15	1.943e-5	15	8122.842	2	NC	1
69		16	max	.001	1	.007	2	.005	1	6.656e-4	1	NC	3	NC	2
70			min	0	10	-.007	3	0	15	2.054e-5	15	6955.762	2	9802.038	1
71		17	max	.001	1	.008	1	.005	1	7.012e-4	1	NC	3	NC	2
72			min	0	10	-.007	3	0	15	2.165e-5	15	6022.417	1	8527.117	1
73		18	max	.001	1	.009	1	.006	1	7.369e-4	1	NC	3	NC	2
74			min	0	10	-.007	3	0	15	2.276e-5	15	5269.378	1	7597.681	1
75		19	max	.001	1	.01	1	.007	1	7.726e-4	1	NC	3	NC	2
76			min	0	10	-.007	3	0	15	2.386e-5	15	4673.114	1	6910.295	1
77	M4	1	max	.003	1	.007	2	0	15	-2.655e-5	15	NC	1	NC	2
78			min	0	3	-.005	3	-.005	1	-8.691e-4	1	NC	1	3880.919	1
79		2	max	.003	1	.007	2	0	15	-2.655e-5	15	NC	1	NC	2
80			min	0	3	-.005	3	-.005	1	-8.691e-4	1	NC	1	4233.78	1
81		3	max	.002	1	.007	2	0	15	-2.655e-5	15	NC	1	NC	2
82			min	0	3	-.005	3	-.004	1	-8.691e-4	1	NC	1	4653.74	1
83		4	max	.002	1	.006	2	0	15	-2.655e-5	15	NC	1	NC	2
84			min	0	3	-.004	3	-.004	1	-8.691e-4	1	NC	1	5158.496	1
85		5	max	.002	1	.006	2	0	15	-2.655e-5	15	NC	1	NC	2
86			min	0	3	-.004	3	-.003	1	-8.691e-4	1	NC	1	5772.159	1
87		6	max	.002	1	.005	2	0	15	-2.655e-5	15	NC	1	NC	2
88			min	0	3	-.004	3	-.003	1	-8.691e-4	1	NC	1	6528.266	1
89		7	max	.002	1	.005	2	0	15	-2.655e-5	15	NC	1	NC	2
90			min	0	3	-.004	3	-.003	1	-8.691e-4	1	NC	1	7474.585	1
91		8	max	.002	1	.004	2	0	15	-2.655e-5	15	NC	1	NC	2
92			min	0	3	-.003	3	-.002	1	-8.691e-4	1	NC	1	8681.072	1
93		9	max	.002	1	.004	2	0	15	-2.655e-5	15	NC	1	NC	1
94			min	0	3	-.003	3	-.002	1	-8.691e-4	1	NC	1	NC	1
95		10	max	.001	1	.004	2	0	15	-2.655e-5	15	NC	1	NC	1
96			min	0	3	-.003	3	-.002	1	-8.691e-4	1	NC	1	NC	1
97		11	max	.001	1	.003	2	0	15	-2.655e-5	15	NC	1	NC	1
98			min	0	3	-.002	3	-.001	1	-8.691e-4	1	NC	1	NC	1
99		12	max	.001	1	.003	2	0	15	-2.655e-5	15	NC	1	NC	1
100			min	0	3	-.002	3	0	1	-8.691e-4	1	NC	1	NC	1
101		13	max	0	1	.002	2	0	15	-2.655e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-8.691e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-2.655e-5	15	NC	1	NC	1
104			min	0	3	-.001	3	0	1	-8.691e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-2.655e-5	15	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-8.691e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-2.655e-5	15	NC	1	NC	1
108			min	0	3	0	3	0	1	-8.691e-4	1	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-2.655e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-8.691e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-2.655e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-8.691e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.655e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-8.691e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.02	2	.004	1	2.067e-4	1	NC	3	NC	2
116			min	-.009	3	-.013	3	-.002	3	3.655e-6	10	1512.057	2	7788.748	1
117		2	max	.01	1	.019	2	.004	1	1.932e-4	1	NC	3	NC	2
118			min	-.008	3	-.013	3	-.002	3	2.936e-6	10	1612.272	2	8444.28	1
119		3	max	.009	1	.017	2	.003	1	1.797e-4	1	NC	3	NC	2
120			min	-.008	3	-.012	3	-.002	3	2.217e-6	10	1726.403	2	9222.607	1
121		4	max	.009	1	.016	2	.003	1	1.662e-4	1	NC	3	NC	1
122			min	-.007	3	-.011	3	-.002	3	1.499e-6	10	1857.222	2	NC	1
123		5	max	.008	1	.015	2	.003	1	1.527e-4	1	NC	3	NC	1
124			min	-.007	3	-.011	3	-.001	3	7.797e-7	10	2008.296	2	NC	1
125		6	max	.008	1	.014	2	.002	1	1.392e-4	1	NC	3	NC	1
126			min	-.006	3	-.01	3	-.001	3	6.092e-8	10	2184.293	2	NC	1
127		7	max	.007	1	.013	2	.002	1	1.274e-4	3	NC	3	NC	1
128			min	-.006	3	-.009	3	-.001	3	-6.579e-7	10	2391.44	2	NC	1
129		8	max	.006	1	.011	2	.002	1	1.239e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.001	3	-1.377e-6	10	2638.23	2	NC	1
131		9	max	.006	1	.01	2	.002	1	1.203e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	0	3	-3.484e-6	2	2936.554	2	NC	1
133		10	max	.005	1	.009	2	.001	1	1.168e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	0	3	-7.649e-6	2	3303.589	2	NC	1
135		11	max	.005	1	.008	2	.001	1	1.133e-4	3	NC	3	NC	1
136			min	-.004	3	-.007	3	0	3	-1.181e-5	2	3765.102	2	NC	1
137		12	max	.004	1	.007	2	0	1	1.097e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	0	3	-1.598e-5	2	4361.577	2	NC	1
139		13	max	.003	1	.006	2	0	1	1.062e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-2.015e-5	2	5160.487	2	NC	1
141		14	max	.003	1	.005	2	0	1	1.027e-4	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-2.431e-5	2	6283.278	2	NC	1
143		15	max	.002	1	.004	2	0	1	9.913e-5	3	NC	3	NC	1
144			min	-.002	3	-.003	3	0	3	-2.848e-5	2	7972.823	2	NC	1
145		16	max	.002	1	.003	2	0	1	9.559e-5	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-3.264e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	9.205e-5	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-3.681e-5	2	NC	1	NC	1
149		18	max	0	1	0	2	0	1	8.852e-5	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-4.097e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	8.498e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.514e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.025e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-3.849e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	1.7e-5	2	NC	1	NC	1
156			min	0	2	-.001	3	0	2	-2.943e-5	3	NC	1	NC	1
157		3	max	0	9	.003	2	0	3	1.515e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-2.036e-5	3	NC	1	NC	1
159		4	max	0	9	.004	2	0	3	1.474e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	2	-1.129e-5	3	NC	1	NC	1
161		5	max	0	9	.005	1	0	3	1.433e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	2	-2.229e-6	3	8974.265	1	NC	1
163		6	max	0	9	.006	1	0	3	1.393e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	3.552e-7	15	7105.389	1	NC	1
165		7	max	0	9	.008	1	0	3	1.59e-5	3	NC	3	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	3.864e-7	15	5833.491	1	NC	1
167		8	max	0	9	.009	1	.001	3	2.497e-5	3	NC	3	NC	1
168			min	0	2	-.01	3	0	1	-2.524e-6	2	4907.464	1	NC	1
169		9	max	0	9	.011	1	.001	3	3.403e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	-5.778e-6	2	4201.691	1	NC	1
171		10	max	0	9	.013	1	.001	3	4.31e-5	3	NC	3	NC	1
172			min	0	2	-.012	3	0	1	-9.031e-6	2	3646.141	1	NC	1
173		11	max	.001	9	.014	1	.001	3	5.216e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	-.001	1	-1.228e-5	2	3198.466	1	NC	1
175		12	max	.001	9	.016	1	.001	3	6.123e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-1.554e-5	2	2831.377	1	NC	1
177		13	max	.001	9	.018	1	.001	3	7.029e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.001	1	-1.879e-5	2	2526.357	1	NC	1
179		14	max	.001	9	.02	1	.001	3	7.936e-5	3	NC	3	NC	1
180			min	-.001	2	-.016	3	-.001	1	-2.205e-5	2	2270.306	1	NC	1
181		15	max	.002	9	.022	1	.001	3	8.843e-5	3	NC	3	NC	1
182			min	-.001	2	-.017	3	-.002	1	-2.53e-5	2	2053.639	1	NC	1
183		16	max	.002	9	.025	1	.001	3	9.749e-5	3	NC	3	NC	1
184			min	-.002	2	-.018	3	-.002	1	-2.855e-5	2	1869.145	1	NC	1
185		17	max	.002	9	.027	1	.001	3	1.066e-4	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-3.181e-5	2	1711.28	1	NC	1
187		18	max	.002	9	.029	1	.001	3	1.156e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	-.002	1	-3.506e-5	2	1575.708	1	NC	1
189		19	max	.002	9	.032	1	0	3	1.247e-4	3	NC	3	NC	1
190			min	-.002	2	-.02	3	-.002	1	-3.831e-5	2	1458.992	1	NC	1
191	M8	1	max	.008	1	.023	2	.002	1	-3.858e-7	10	NC	1	NC	2
192			min	-.002	3	-.015	3	0	3	-1.004e-4	3	NC	1	8711.121	1
193		2	max	.008	1	.022	2	.002	1	-3.858e-7	10	NC	1	NC	2
194			min	-.002	3	-.014	3	0	3	-1.004e-4	3	NC	1	9497.512	1
195		3	max	.007	1	.021	2	.002	1	-3.858e-7	10	NC	1	NC	1
196			min	-.002	3	-.013	3	0	3	-1.004e-4	3	NC	1	NC	1
197		4	max	.007	1	.019	2	.002	1	-3.858e-7	10	NC	1	NC	1
198			min	-.002	3	-.012	3	0	3	-1.004e-4	3	NC	1	NC	1
199		5	max	.006	1	.018	2	.001	1	-3.858e-7	10	NC	1	NC	1
200			min	-.001	3	-.012	3	0	3	-1.004e-4	3	NC	1	NC	1
201		6	max	.006	1	.017	2	.001	1	-3.858e-7	10	NC	1	NC	1
202			min	-.001	3	-.011	3	0	3	-1.004e-4	3	NC	1	NC	1
203		7	max	.006	1	.015	2	.001	1	-3.858e-7	10	NC	1	NC	1
204			min	-.001	3	-.01	3	0	3	-1.004e-4	3	NC	1	NC	1
205		8	max	.005	1	.014	2	0	1	-3.858e-7	10	NC	1	NC	1
206			min	-.001	3	-.009	3	0	3	-1.004e-4	3	NC	1	NC	1
207		9	max	.005	1	.013	2	0	1	-3.858e-7	10	NC	1	NC	1
208			min	-.001	3	-.008	3	0	3	-1.004e-4	3	NC	1	NC	1
209		10	max	.004	1	.012	2	0	1	-3.858e-7	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.004e-4	3	NC	1	NC	1
211		11	max	.004	1	.01	2	0	1	-3.858e-7	10	NC	1	NC	1
212			min	0	3	-.007	3	0	3	-1.004e-4	3	NC	1	NC	1
213		12	max	.003	1	.009	2	0	1	-3.858e-7	10	NC	1	NC	1
214			min	0	3	-.006	3	0	3	-1.004e-4	3	NC	1	NC	1
215		13	max	.003	1	.008	2	0	1	-3.858e-7	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.004e-4	3	NC	1	NC	1
217		14	max	.002	1	.006	2	0	1	-3.858e-7	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.004e-4	3	NC	1	NC	1
219		15	max	.002	1	.005	2	0	1	-3.858e-7	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.004e-4	3	NC	1	NC	1
221		16	max	.001	1	.004	2	0	1	-3.858e-7	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.004e-4	3	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-3.858e-7	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.004e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-3.858e-7	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.004e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-3.858e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.004e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.006	2	0	3	8.804e-4	1	NC	3	NC	1
230			min	-.003	3	-.005	3	-.002	1	-1.575e-4	3	4829.734	2	NC	1
231		2	max	.003	1	.006	2	0	3	8.349e-4	1	NC	3	NC	1
232			min	-.003	3	-.005	3	-.002	1	-1.534e-4	3	5212.083	2	NC	1
233		3	max	.003	1	.005	2	0	3	7.893e-4	1	NC	3	NC	1
234			min	-.002	3	-.005	3	-.002	1	-1.494e-4	3	5657.237	2	NC	1
235		4	max	.003	1	.005	2	0	3	7.438e-4	1	NC	3	NC	1
236			min	-.002	3	-.004	3	-.001	1	-1.453e-4	3	6178.727	2	NC	1
237		5	max	.003	1	.004	2	0	3	6.983e-4	1	NC	3	NC	1
238			min	-.002	3	-.004	3	-.001	1	-1.412e-4	3	6794.173	2	NC	1
239		6	max	.002	1	.004	2	0	3	6.528e-4	1	NC	3	NC	1
240			min	-.002	3	-.004	3	-.001	1	-1.371e-4	3	7526.881	2	NC	1
241		7	max	.002	1	.004	2	0	3	6.073e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	-.001	1	-1.331e-4	3	8408.249	2	NC	1
243		8	max	.002	1	.003	2	0	3	5.618e-4	1	NC	1	NC	1
244			min	-.002	3	-.004	3	0	1	-1.29e-4	3	9481.5	2	NC	1
245		9	max	.002	1	.003	2	0	3	5.163e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-1.249e-4	3	NC	1	NC	1
247		10	max	.002	1	.002	2	0	3	4.708e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-1.208e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.253e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-1.168e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	3.798e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.127e-4	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	3.343e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-1.086e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	2.888e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.045e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.433e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.004e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.978e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-9.637e-5	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.523e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-9.229e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.068e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-8.822e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.127e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.414e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	3.835e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.928e-5	1	NC	1	NC	1
269		2	max	0	1	0	2	0	2	2.762e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-9.14e-5	1	NC	1	NC	1
271		3	max	0	1	0	2	0	2	1.689e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-1.535e-4	1	NC	1	NC	1
273		4	max	0	1	0	2	0	2	6.154e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-2.156e-4	1	NC	1	NC	1
275		5	max	0	1	0	2	0	10	-3.396e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	0	1	-2.778e-4	1	NC	1	NC	1
277		6	max	0	1	0	2	0	15	-1.022e-5	12	NC	1	NC	1
278			min	0	10	-.004	3	-.002	1	-3.399e-4	1	NC	1	NC	1
279		7	max	0	1	.001	2	0	15	-1.235e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-.004	3	-.002	1	-4.02e-4	1	NC	1	NC	1
281		8	max	0	1	.001	2	0	15	-1.429e-5	15	NC	1	NC	1
282			min	0	10	-.005	3	-.003	1	-4.641e-4	1	NC	1	NC	1
283		9	max	0	1	.002	2	0	15	-1.624e-5	15	NC	1	NC	1
284			min	0	10	-.005	3	-.004	1	-5.262e-4	1	NC	1	NC	1
285		10	max	0	1	.002	2	0	15	-1.818e-5	15	NC	1	NC	2
286			min	0	10	-.006	3	-.005	1	-5.883e-4	1	NC	1	9177.134	1
287		11	max	0	1	.003	2	0	15	-2.013e-5	15	NC	1	NC	2
288			min	0	10	-.006	3	-.006	1	-6.505e-4	1	NC	1	7627.109	1
289		12	max	0	1	.003	2	0	15	-2.207e-5	15	NC	1	NC	2
290			min	0	10	-.006	3	-.007	1	-7.126e-4	1	NC	1	6503.696	1
291		13	max	0	1	.004	2	0	15	-2.402e-5	15	NC	1	NC	2
292			min	0	10	-.007	3	-.008	1	-7.747e-4	1	NC	1	5665.773	1
293		14	max	.001	1	.005	2	0	15	-2.596e-5	15	NC	3	NC	2
294			min	0	10	-.007	3	-.009	1	-8.368e-4	1	9410.903	2	5027.148	1
295		15	max	.001	1	.006	2	0	15	-2.791e-5	15	NC	3	NC	2
296			min	0	10	-.007	3	-.01	1	-8.989e-4	1	8001.606	2	4532.827	1
297		16	max	.001	1	.007	2	0	15	-2.985e-5	15	NC	3	NC	2
298			min	0	10	-.007	3	-.011	1	-9.611e-4	1	6889.311	2	4146.507	1
299		17	max	.001	1	.008	1	0	15	-3.179e-5	15	NC	3	NC	2
300			min	0	10	-.007	3	-.012	1	-1.023e-3	1	5977.908	1	3843.62	1
301		18	max	.001	1	.009	1	0	15	-3.374e-5	15	NC	3	NC	2
302			min	0	10	-.007	3	-.013	1	-1.085e-3	1	5253.855	1	3607.293	1
303		19	max	.001	1	.01	1	0	15	-3.568e-5	15	NC	3	NC	3
304			min	0	10	-.007	3	-.013	1	-1.147e-3	1	4675.901	1	3425.94	1
305	M12	1	max	.003	1	.007	2	.011	1	9.781e-4	1	NC	1	NC	3
306			min	0	3	-.005	3	0	15	3.083e-5	15	NC	1	1738.462	1
307		2	max	.003	1	.007	2	.01	1	9.781e-4	1	NC	1	NC	3
308			min	0	3	-.005	3	0	15	3.083e-5	15	NC	1	1895.919	1
309		3	max	.002	1	.007	2	.009	1	9.781e-4	1	NC	1	NC	3
310			min	0	3	-.005	3	0	15	3.083e-5	15	NC	1	2083.347	1
311		4	max	.002	1	.006	2	.008	1	9.781e-4	1	NC	1	NC	3
312			min	0	3	-.004	3	0	15	3.083e-5	15	NC	1	2308.643	1
313		5	max	.002	1	.006	2	.007	1	9.781e-4	1	NC	1	NC	3
314			min	0	3	-.004	3	0	15	3.083e-5	15	NC	1	2582.572	1
315		6	max	.002	1	.005	2	.007	1	9.781e-4	1	NC	1	NC	3
316			min	0	3	-.004	3	0	15	3.083e-5	15	NC	1	2920.1	1
317		7	max	.002	1	.005	2	.006	1	9.781e-4	1	NC	1	NC	3
318			min	0	3	-.004	3	0	15	3.083e-5	15	NC	1	3342.549	1
319		8	max	.002	1	.004	2	.005	1	9.781e-4	1	NC	1	NC	2
320			min	0	3	-.003	3	0	15	3.083e-5	15	NC	1	3881.143	1
321		9	max	.002	1	.004	2	.004	1	9.781e-4	1	NC	1	NC	2
322			min	0	3	-.003	3	0	15	3.083e-5	15	NC	1	4583.109	1
323		10	max	.001	1	.004	2	.003	1	9.781e-4	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	3.083e-5	15	NC	1	5522.702	1
325		11	max	.001	1	.003	2	.003	1	9.781e-4	1	NC	1	NC	2
326			min	0	3	-.002	3	0	15	3.083e-5	15	NC	1	6822.227	1
327		12	max	.001	1	.003	2	.002	1	9.781e-4	1	NC	1	NC	2
328			min	0	3	-.002	3	0	15	3.083e-5	15	NC	1	8695.052	1
329		13	max	0	1	.002	2	.002	1	9.781e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	3.083e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	.001	1	9.781e-4	1	NC	1	NC	1
332			min	0	3	-.001	3	0	15	3.083e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	9.781e-4	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	3.083e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	9.781e-4	1	NC	1	NC	1
336			min	0	3	0	3	0	15	3.083e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	9.781e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.083e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	9.781e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.083e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	9.781e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.083e-5	15	NC	1	NC	1
343	M1	1	max	.005	3	.022	3	.001	3	2.007e-2	1	NC	1	NC	1
344			min	-.006	2	-.031	1	-.004	1	-1.612e-2	3	NC	1	NC	1
345		2	max	.005	3	.012	3	0	3	9.657e-3	1	NC	4	NC	2
346			min	-.006	2	-.017	1	-.01	1	-7.977e-3	3	3136.687	1	8863.661	1
347		3	max	.005	3	.002	3	0	3	1.915e-5	3	NC	5	NC	2
348			min	-.006	2	-.003	1	-.013	1	-5.592e-4	1	1622.445	1	5373.33	1
349		4	max	.005	3	.009	1	0	3	2.159e-5	3	NC	5	NC	2
350			min	-.006	2	-.006	3	-.015	1	-4.597e-4	1	1148.091	1	4444.232	1
351		5	max	.005	3	.019	1	0	3	2.404e-5	3	NC	5	NC	2
352			min	-.006	2	-.012	3	-.015	1	-3.603e-4	1	920.188	1	4264.484	1
353		6	max	.005	3	.027	1	0	3	2.649e-5	3	NC	5	NC	2
354			min	-.006	2	-.017	3	-.014	1	-2.609e-4	1	791.466	1	4558.115	1
355		7	max	.005	3	.033	1	0	3	2.894e-5	3	NC	5	NC	2
356			min	-.006	2	-.021	3	-.013	1	-1.614e-4	1	713.631	1	5417.491	1
357		8	max	.005	3	.038	1	0	3	3.138e-5	3	NC	5	NC	2
358			min	-.006	2	-.024	3	-.01	1	-6.198e-5	1	666.607	1	7410.372	1
359		9	max	.005	3	.041	1	0	3	3.746e-5	1	NC	5	NC	1
360			min	-.006	2	-.025	3	-.007	1	1.397e-6	15	641.026	1	NC	1
361		10	max	.005	3	.041	1	0	3	1.369e-4	1	NC	5	NC	1
362			min	-.006	2	-.025	3	-.004	1	4.443e-6	15	632.578	1	NC	1
363		11	max	.005	3	.04	1	0	3	2.363e-4	1	NC	5	NC	1
364			min	-.006	2	-.024	3	-.001	1	7.49e-6	15	639.965	1	NC	1
365		12	max	.005	3	.038	1	.002	1	3.358e-4	1	NC	5	NC	2
366			min	-.006	2	-.022	3	0	15	1.054e-5	15	664.369	1	8703.524	1
367		13	max	.005	3	.033	1	.004	1	4.352e-4	1	NC	5	NC	2
368			min	-.006	2	-.019	3	0	15	1.358e-5	15	709.942	1	6027.885	1
369		14	max	.005	3	.026	1	.006	1	5.347e-4	1	NC	5	NC	2
370			min	-.006	2	-.015	3	0	15	1.663e-5	15	785.773	1	4935.858	1
371		15	max	.005	3	.018	1	.007	1	6.341e-4	1	NC	5	NC	2
372			min	-.007	2	-.011	3	0	15	1.967e-5	15	911.34	1	4542.356	1
373		16	max	.005	3	.008	1	.007	1	7.074e-4	1	NC	5	NC	2
374			min	-.007	2	-.005	3	0	15	2.193e-5	15	1133.077	1	4678.293	1
375		17	max	.005	3	.002	3	.005	1	1.577e-4	1	NC	5	NC	2
376			min	-.007	2	-.004	1	0	15	5.452e-6	15	1590.459	1	5609.728	1
377		18	max	.005	3	.009	3	.002	1	1.124e-2	1	NC	4	NC	2
378			min	-.007	2	-.018	1	0	15	-3.802e-3	3	3065.63	1	9201.002	1
379		19	max	.005	3	.017	3	0	3	2.254e-2	1	NC	1	NC	1
380			min	-.007	2	-.034	1	-.003	1	-7.704e-3	3	NC	1	NC	1
381	M5	1	max	.014	3	.066	3	.001	3	4.193e-7	3	NC	1	NC	1
382			min	-.02	2	-.094	1	-.005	1	3.545e-8	15	NC	1	NC	1
383		2	max	.014	3	.036	3	.002	3	3.79e-5	3	NC	5	NC	1
384			min	-.02	2	-.05	1	-.004	1	-9.344e-5	1	1059.765	1	NC	1
385		3	max	.014	3	.008	3	.002	3	7.468e-5	3	NC	5	NC	1
386			min	-.02	2	-.009	1	-.004	1	-1.853e-4	1	545.755	1	NC	1
387		4	max	.014	3	.026	1	.002	3	7.414e-5	3	NC	5	NC	1
388			min	-.02	2	-.015	3	-.003	1	-1.73e-4	1	385.397	1	NC	1
389		5	max	.014	3	.056	1	.003	3	7.36e-5	3	NC	15	NC	1
390			min	-.02	2	-.034	3	-.003	1	-1.607e-4	1	308.339	1	NC	1
391		6	max	.014	3	.081	1	.003	3	7.305e-5	3	NC	15	NC	1
392			min	-.02	2	-.049	3	-.003	1	-1.484e-4	1	264.767	1	NC	1
393		7	max	.014	3	.1	1	.003	3	7.251e-5	3	NC	15	NC	1



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Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394		min	-.02	2	-.06	3	-.002	1	-1.361e-4	1	238.358	1	NC	1
395	8	max	.014	3	.113	1	.003	3	7.197e-5	3	NC	15	NC	1
396		min	-.02	2	-.068	3	-.002	1	-1.237e-4	1	222.324	1	NC	1
397	9	max	.014	3	.122	1	.003	3	7.142e-5	3	9797.938	15	NC	1
398		min	-.02	2	-.072	3	-.002	1	-1.114e-4	1	213.496	1	NC	1
399	10	max	.014	3	.124	1	.003	3	7.088e-5	3	9698.065	15	NC	1
400		min	-.02	2	-.072	3	-.002	1	-9.911e-5	1	210.41	1	NC	1
401	11	max	.014	3	.122	1	.002	3	7.033e-5	3	9839.523	15	NC	1
402		min	-.02	2	-.07	3	-.002	1	-8.68e-5	1	212.614	1	NC	1
403	12	max	.014	3	.113	1	.002	3	6.979e-5	3	NC	15	NC	1
404		min	-.02	2	-.064	3	-.002	1	-7.449e-5	1	220.487	1	NC	1
405	13	max	.014	3	.099	1	.002	3	6.925e-5	3	NC	15	NC	1
406		min	-.021	2	-.055	3	-.002	1	-6.217e-5	1	235.402	1	NC	1
407	14	max	.014	3	.08	1	.002	3	6.87e-5	3	NC	15	NC	1
408		min	-.021	2	-.044	3	-.002	1	-4.986e-5	1	260.38	1	NC	1
409	15	max	.014	3	.054	1	.001	3	6.816e-5	3	NC	15	NC	1
410		min	-.021	2	-.03	3	-.002	1	-3.755e-5	1	301.919	1	NC	1
411	16	max	.014	3	.023	1	0	3	6.598e-5	3	NC	5	NC	1
412		min	-.021	2	-.013	3	-.002	1	-3.377e-5	2	375.599	1	NC	1
413	17	max	.014	3	.005	3	0	3	2.478e-5	3	NC	5	NC	1
414		min	-.021	2	-.013	1	-.002	1	-1.868e-4	1	528.915	1	NC	1
415	18	max	.014	3	.026	3	0	3	1.208e-5	3	NC	5	NC	1
416		min	-.021	2	-.056	1	-.002	1	-9.577e-5	1	1024.479	1	NC	1
417	19	max	.014	3	.048	3	0	3	0	5	NC	1	NC	1
418		min	-.021	2	-.103	1	-.002	1	-8.372e-8	3	NC	1	NC	1
419	M9	1	max	.005	.022	3	0	3	1.612e-2	3	NC	1	NC	1
420		min	-.006	2	-.031	1	-.006	1	-2.007e-2	1	NC	1	NC	1
421	2	max	.005	3	.012	3	0	3	7.997e-3	3	NC	4	NC	1
422		min	-.006	2	-.017	1	-.001	1	-9.916e-3	1	3137.535	1	NC	1
423	3	max	.005	3	.002	3	.002	1	4.547e-5	1	NC	5	NC	2
424		min	-.006	2	-.003	1	0	3	1.619e-6	15	1622.897	1	6433.157	1
425	4	max	.005	3	.009	1	.003	1	1.084e-5	3	NC	5	NC	2
426		min	-.006	2	-.006	3	0	3	-3.712e-5	1	1148.412	1	5457.094	1
427	5	max	.005	3	.019	1	.004	1	2.113e-6	3	NC	5	NC	2
428		min	-.006	2	-.012	3	0	3	-1.197e-4	1	920.437	1	5415.619	1
429	6	max	.005	3	.027	1	.003	1	-4.499e-6	12	NC	5	NC	2
430		min	-.006	2	-.017	3	-.001	3	-2.023e-4	1	791.671	1	6089.689	1
431	7	max	.005	3	.033	1	.001	1	-8.612e-6	15	NC	5	NC	2
432		min	-.006	2	-.021	3	-.001	3	-2.849e-4	1	713.805	1	7902.439	1
433	8	max	.005	3	.038	1	0	2	-1.117e-5	15	NC	5	NC	1
434		min	-.006	2	-.024	3	-.002	3	-3.675e-4	1	666.759	1	NC	1
435	9	max	.005	3	.041	1	0	10	-1.373e-5	15	NC	5	NC	1
436		min	-.006	2	-.025	3	-.003	1	-4.501e-4	1	641.163	1	NC	1
437	10	max	.005	3	.041	1	0	15	-1.629e-5	15	NC	5	NC	1
438		min	-.006	2	-.025	3	-.006	1	-5.327e-4	1	632.702	1	NC	1
439	11	max	.005	3	.04	1	0	15	-1.884e-5	15	NC	5	NC	2
440		min	-.006	2	-.024	3	-.009	1	-6.153e-4	1	640.081	1	9536.03	1
441	12	max	.005	3	.038	1	0	15	-2.14e-5	15	NC	5	NC	2
442		min	-.006	2	-.022	3	-.011	1	-6.979e-4	1	664.477	1	6249.606	1
443	13	max	.005	3	.033	1	0	15	-2.396e-5	15	NC	5	NC	2
444		min	-.006	2	-.019	3	-.013	1	-7.805e-4	1	710.046	1	4880.063	1
445	14	max	.005	3	.026	1	0	15	-2.652e-5	15	NC	5	NC	2
446		min	-.006	2	-.015	3	-.014	1	-8.631e-4	1	785.876	1	4255.262	1
447	15	max	.005	3	.018	1	0	15	-2.907e-5	15	NC	5	NC	2
448		min	-.006	2	-.011	3	-.014	1	-9.457e-4	1	911.446	1	4070.084	1
449	16	max	.005	3	.008	1	0	15	-3.098e-5	15	NC	5	NC	2
450		min	-.007	2	-.005	3	-.013	1	-1.008e-3	1	1133.193	1	4301.575	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	15	-1.737e-5	15	NC	5	NC	2
452			min	-.007	2	-.004	1	-.011	1	-5.829e-4	1	1590.612	1	5253.662	1
453		18	max	.005	3	.009	3	0	15	3.803e-3	3	NC	4	NC	2
454			min	-.007	2	-.018	1	-.007	1	-1.145e-2	1	3065.911	1	8735.048	1
455		19	max	.005	3	.017	3	0	3	7.704e-3	3	NC	1	NC	1
456			min	-.007	2	-.034	1	-.002	1	-2.254e-2	1	NC	1	NC	1
457	M13	1	max	.006	1	.022	3	.005	3	3.926e-3	3	NC	1	NC	1
458			min	0	3	-.031	1	-.006	2	-5.641e-3	1	NC	1	NC	1
459		2	max	.006	1	.193	3	.04	1	4.751e-3	3	NC	5	NC	2
460			min	0	3	-.245	1	0	10	-6.849e-3	1	929.194	1	4398.509	1
461		3	max	.006	1	.333	3	.103	1	5.576e-3	3	NC	5	NC	3
462			min	0	3	-.419	1	.003	15	-8.056e-3	1	511.151	1	1837.188	1
463		4	max	.006	1	.421	3	.156	1	6.4e-3	3	NC	5	NC	3
464			min	0	3	-.528	1	.005	15	-9.264e-3	1	398.665	1	1230.63	1
465		5	max	.006	1	.445	3	.182	1	7.225e-3	3	NC	5	NC	3
466			min	0	3	-.56	1	.006	15	-1.047e-2	1	374.559	1	1059.638	1
467		6	max	.005	1	.409	3	.173	1	8.05e-3	3	NC	5	NC	3
468			min	0	3	-.516	1	.005	15	-1.168e-2	1	408.493	1	1112.312	1
469		7	max	.005	1	.324	3	.132	1	8.875e-3	3	NC	5	NC	3
470			min	0	3	-.412	1	.004	10	-1.289e-2	1	520.814	1	1449.962	1
471		8	max	.005	1	.213	3	.07	1	9.7e-3	3	NC	5	NC	3
472			min	0	3	-.276	1	-.003	10	-1.409e-2	1	811.185	1	2655.209	1
473		9	max	.005	1	.112	3	.013	3	1.052e-2	3	NC	5	NC	1
474			min	0	3	-.151	1	-.009	2	-1.53e-2	1	1659.393	1	NC	1
475		10	max	.005	1	.066	3	.014	3	1.135e-2	3	NC	4	NC	1
476			min	-.001	3	-.094	1	-.02	2	-1.651e-2	1	3165.212	1	NC	1
477		11	max	.005	1	.112	3	.016	3	1.052e-2	3	NC	5	NC	1
478			min	-.001	3	-.151	1	-.008	2	-1.53e-2	1	1659.393	1	NC	1
479		12	max	.005	1	.213	3	.076	1	9.7e-3	3	NC	5	NC	3
480			min	-.001	3	-.276	1	-.002	10	-1.409e-2	1	811.185	1	2450.171	1
481		13	max	.005	1	.324	3	.139	1	8.876e-3	3	NC	5	NC	3
482			min	-.001	3	-.412	1	.004	10	-1.289e-2	1	520.814	1	1375.045	1
483		14	max	.005	1	.409	3	.181	1	8.051e-3	3	NC	5	NC	3
484			min	-.001	3	-.516	1	.006	15	-1.168e-2	1	408.493	1	1066.659	1
485		15	max	.005	1	.446	3	.189	1	7.227e-3	3	NC	5	NC	3
486			min	-.001	3	-.56	1	.006	15	-1.047e-2	1	374.559	1	1021.53	1
487		16	max	.005	1	.421	3	.162	1	6.402e-3	3	NC	5	NC	3
488			min	-.001	3	-.528	1	.005	15	-9.264e-3	1	398.666	1	1188.594	1
489		17	max	.004	1	.333	3	.107	1	5.577e-3	3	NC	5	NC	3
490			min	-.001	3	-.419	1	.003	15	-8.056e-3	1	511.152	1	1771.883	1
491		18	max	.004	1	.193	3	.042	1	4.753e-3	3	NC	5	NC	2
492			min	-.001	3	-.244	1	0	10	-6.848e-3	1	929.195	1	4210.453	1
493		19	max	.004	1	.022	3	.005	3	3.928e-3	3	NC	1	NC	1
494			min	-.001	3	-.031	1	-.006	2	-5.641e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.017	3	.005	3	5.868e-3	1	NC	1	NC	1
496			min	0	3	-.034	1	-.007	2	-2.948e-3	3	NC	1	NC	1
497		2	max	.002	1	.099	3	.043	1	7.153e-3	1	NC	5	NC	2
498			min	0	3	-.273	1	0	10	-3.539e-3	3	827.425	1	4125.696	1
499		3	max	.002	1	.166	3	.108	1	8.439e-3	1	NC	5	NC	3
500			min	0	3	-.469	1	.003	15	-4.13e-3	3	455.207	1	1754.66	1
501		4	max	.002	1	.208	3	.162	1	9.724e-3	1	NC	5	NC	3
502			min	0	3	-.591	1	.005	15	-4.721e-3	3	355.089	1	1184.055	1
503		5	max	.002	1	.221	3	.188	1	1.101e-2	1	NC	15	NC	3
504			min	0	3	-.627	1	.006	15	-5.313e-3	3	333.707	1	1022.388	1
505		6	max	.002	1	.206	3	.179	1	1.229e-2	1	NC	5	NC	3
506			min	0	3	-.578	1	.006	15	-5.904e-3	3	364.112	1	1072.786	1
507		7	max	.002	1	.167	3	.137	1	1.358e-2	1	NC	5	NC	3





Company : Schletter, Inc.
 Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	3	-.012	15	.016	1	8.308e-3	3	8697.535	15	NC	4
566		min	0	10	-.052	4	-.011	3	-1.247e-2	1	2044.486	4	3045.541	1
567	18	max	0	3	-.006	15	.002	9	8.81e-3	3	NC	5	NC	4
568		min	0	10	-.027	4	-.005	2	-1.325e-2	1	4017.73	4	5418.397	1
569	19	max	0	3	.005	3	.016	3	9.311e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.021	2	-1.402e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.005	3	3.147e-3	3	NC	1	NC	1
572		min	0	1	-.002	1	-.007	2	-4.427e-3	1	NC	1	NC	1
573	2	max	0	10	-.006	15	.006	1	3.002e-3	3	NC	5	NC	2
574		min	0	1	-.026	4	0	10	-4.209e-3	1	4017.73	4	9497.54	1
575	3	max	0	10	-.012	15	.014	1	2.858e-3	3	8697.535	15	NC	3
576		min	0	1	-.051	4	-.004	3	-3.991e-3	1	2044.486	4	5369.27	1
577	4	max	0	10	-.017	15	.021	1	2.713e-3	3	5967.017	15	NC	4
578		min	0	1	-.074	4	-.008	3	-3.772e-3	1	1402.637	4	4079.896	1
579	5	max	0	10	-.022	15	.025	1	2.568e-3	3	4656.124	15	NC	4
580		min	0	1	-.095	4	-.01	3	-3.554e-3	1	1094.492	4	3519.782	1
581	6	max	0	10	-.026	15	.028	1	2.423e-3	3	3918.621	15	NC	4
582		min	0	1	-.113	4	-.011	3	-3.336e-3	1	921.13	4	3273.344	1
583	7	max	0	10	-.03	15	.028	1	2.279e-3	3	3475.109	15	NC	4
584		min	0	1	-.127	4	-.012	3	-3.118e-3	1	816.876	4	3210.136	1
585	8	max	0	10	-.032	15	.028	1	2.134e-3	3	3208.936	15	NC	4
586		min	0	1	-.138	4	-.012	3	-2.899e-3	1	754.308	4	3285.21	1
587	9	max	0	10	-.034	15	.026	1	1.989e-3	3	3065.666	15	NC	4
588		min	0	1	-.144	4	-.011	3	-2.681e-3	1	720.631	4	3491.835	1
589	10	max	0	10	-.034	15	.024	1	1.844e-3	3	3020.342	15	NC	4
590		min	0	1	-.146	4	-.01	3	-2.463e-3	1	709.977	4	3850.344	1
591	11	max	0	10	-.034	15	.021	1	1.699e-3	3	3065.666	15	NC	4
592		min	0	1	-.144	4	-.009	3	-2.245e-3	1	720.631	4	4412.303	1
593	12	max	0	10	-.032	15	.017	1	1.555e-3	3	3208.936	15	NC	3
594		min	0	1	-.137	4	-.007	3	-2.026e-3	1	754.308	4	5280.151	1
595	13	max	0	10	-.03	15	.014	1	1.41e-3	3	3475.109	15	NC	2
596		min	0	1	-.127	4	-.006	3	-1.808e-3	1	816.876	4	6657.523	1
597	14	max	0	10	-.026	15	.01	1	1.265e-3	3	3918.621	15	NC	2
598		min	0	1	-.113	4	-.004	3	-1.59e-3	1	921.13	4	8982.152	1
599	15	max	0	10	-.022	15	.007	1	1.12e-3	3	4656.124	15	NC	1
600		min	0	1	-.095	4	-.003	3	-1.372e-3	1	1094.492	4	NC	1
601	16	max	0	10	-.017	15	.004	1	9.757e-4	3	5967.017	15	NC	1
602		min	0	1	-.074	4	-.001	3	-1.153e-3	1	1402.637	4	NC	1
603	17	max	0	10	-.012	15	.001	1	8.31e-4	3	8697.535	15	NC	1
604		min	0	1	-.051	4	0	3	-9.351e-4	1	2044.486	4	NC	1
605	18	max	0	10	-.006	15	0	4	6.862e-4	3	NC	5	NC	1
606		min	0	1	-.026	4	0	10	-7.169e-4	1	4017.73	4	NC	1
607	19	max	0	1	0	1	0	1	5.415e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.193e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$ (Eq. D-7)

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ (Sec. D.4.1 & Eq. D-5)

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$ (Eq. D-16f)

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$ (Sec. D.4.1 & Eq. D-16b)

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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