

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	129 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.829 k-ft
$M_z$ =	0.379 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>99%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.716 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.204 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>79%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.566 k-ft
$P_n$ =	0.666 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>43%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

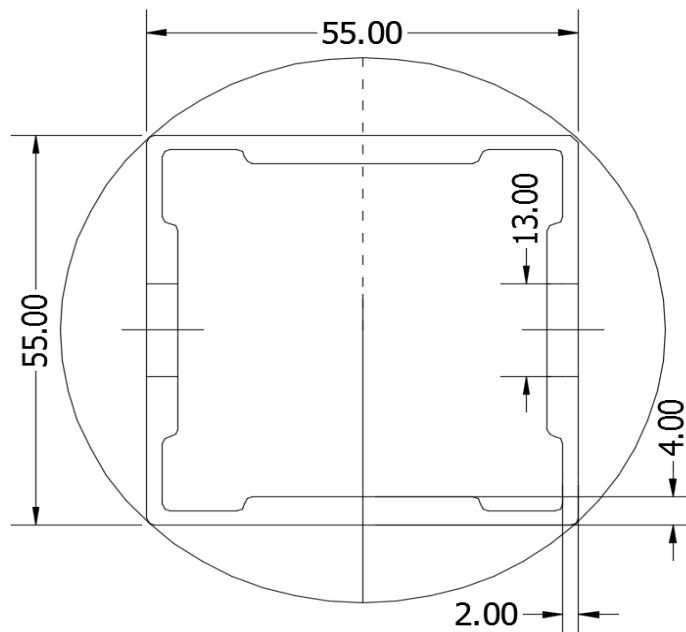
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.583 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>22%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	63.42 in
$\Phi F_{ty \text{ AXIAL}}$ =	12.77 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.008 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.121 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	12.545 k
Utilization =	<b>25%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

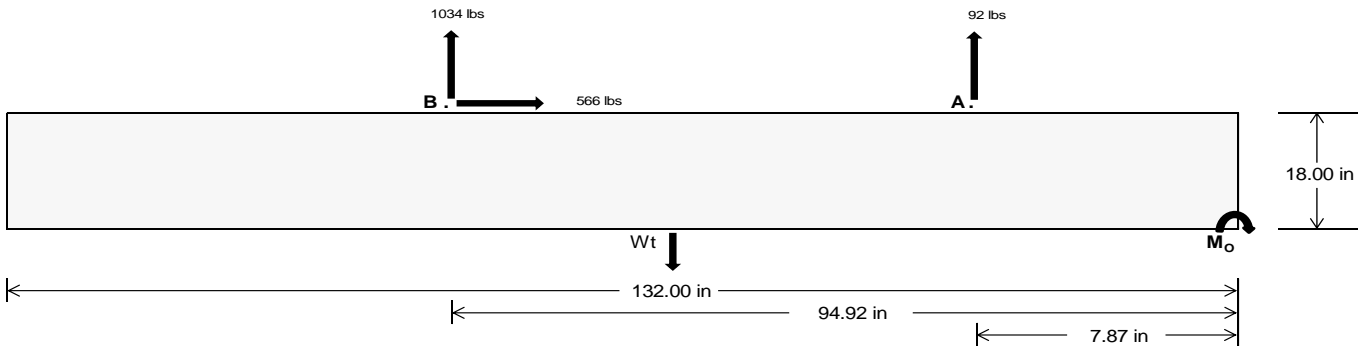
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>420.12</u>	<u>4504.12</u>	k
Compressive Load =	<u>3871.71</u>	<u>4350.88</u>	k
Lateral Load =	<u>382.62</u>	<u>2453.53</u>	k
Moment (Weak Axis) =	<u>0.76</u>	<u>0.35</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 109082.7$  in-lbs  
Resisting Force Required = 1652.77 lbs  
S.F. = 1.67  
Weight Required = 2754.61 lbs  
Minimum Width = 23 in  
Weight Provided = 4585.63 lbs

### Sliding

Force = 565.62 lbs  
Friction = 0.4  
Weight Required = 1414.04 lbs  
Resisting Weight = 4585.63 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 565.62 lbs  
Cohesion = 130 psf  
Area = 21.08 ft<sup>2</sup>  
Resisting = 2292.81 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 132in long x 23in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 23in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 23in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(1.92 \text{ ft}) =$   
23 in 24 in 25 in 26 in  
4586 lbs 4785 lbs 4984 lbs 5184 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in
$F_A$	1489 lbs	1489 lbs	1489 lbs	1489 lbs	1125 lbs	1125 lbs	1125 lbs	1125 lbs	1823 lbs	1823 lbs	1823 lbs	1823 lbs	-183 lbs	-183 lbs	-183 lbs	-183 lbs
$F_B$	1481 lbs	1481 lbs	1481 lbs	1481 lbs	1575 lbs	1575 lbs	1575 lbs	1575 lbs	2155 lbs	2155 lbs	2155 lbs	2155 lbs	-2069 lbs	-2069 lbs	-2069 lbs	-2069 lbs
$F_V$	204 lbs	204 lbs	204 lbs	204 lbs	1036 lbs	1036 lbs	1036 lbs	1036 lbs	913 lbs	913 lbs	913 lbs	913 lbs	-1131 lbs	-1131 lbs	-1131 lbs	-1131 lbs
$P_{total}$	7556 lbs	7755 lbs	7955 lbs	8154 lbs	7286 lbs	7485 lbs	7684 lbs	7884 lbs	8564 lbs	8763 lbs	8962 lbs	9162 lbs	499 lbs	619 lbs	739 lbs	858 lbs
$M$	3949 lbs-ft	3949 lbs-ft	3949 lbs-ft	3949 lbs-ft	3208 lbs-ft	3208 lbs-ft	3208 lbs-ft	3208 lbs-ft	5003 lbs-ft	5003 lbs-ft	5003 lbs-ft	5003 lbs-ft	2400 lbs-ft	2400 lbs-ft	2400 lbs-ft	2400 lbs-ft
$e$	0.52 ft	0.51 ft	0.50 ft	0.48 ft	0.44 ft	0.43 ft	0.42 ft	0.41 ft	0.58 ft	0.57 ft	0.56 ft	0.55 ft	4.81 ft	3.88 ft	3.25 ft	2.80 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	256.2 psf	254.6 psf	253.1 psf	251.7 psf	262.6 psf	260.7 psf	259.0 psf	257.4 psf	276.7 psf	274.3 psf	272.0 psf	269.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	460.6 psf	450.4 psf	441.1 psf	432.5 psf	428.6 psf	419.8 psf	411.7 psf	404.2 psf	535.6 psf	522.4 psf	510.2 psf	498.9 psf	250.8 psf	127.2 psf	105.0 psf	97.7 psf

Maximum Bearing Pressure = 536 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

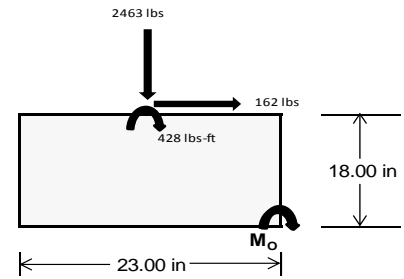
### Overturning Check

$M_o = 1689.2$  ft-lbs  
 Resisting Force Required = 1762.60 lbs  
 S.F. = 1.67  
 Weight Required = 2937.67 lbs  
 Minimum Width = **23 in**  
 Weight Provided = 4585.63 lbs

*A minimum 132in long x 23in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	23 in			23 in			23 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	294 lbs	680 lbs	225 lbs	878 lbs	2463 lbs	825 lbs	110 lbs	199 lbs	42 lbs
$F_v$	226 lbs	222 lbs	230 lbs	166 lbs	162 lbs	179 lbs	227 lbs	223 lbs	228 lbs
$P_{total}$	5971 lbs	6357 lbs	5902 lbs	6282 lbs	7868 lbs	6229 lbs	1770 lbs	1859 lbs	1702 lbs
$M$	896 lbs-ft	887 lbs-ft	908 lbs-ft	671 lbs-ft	672 lbs-ft	713 lbs-ft	895 lbs-ft	885 lbs-ft	899 lbs-ft
$e$	0.15 ft	0.14 ft	0.15 ft	0.11 ft	0.09 ft	0.11 ft	0.51 ft	0.48 ft	0.53 ft
$L/6$	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft
$f_{min}$	150.1 psf	169.9 psf	145.2 psf	198.4 psf	273.4 psf	189.6 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	416.3 psf	433.2 psf	414.7 psf	397.6 psf	472.9 psf	401.3 psf	237.0 psf	233.5 psf	239.8 psf



Maximum Bearing Pressure = 473 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 23in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.441 k
Allowable Uplift =	1.214 k
Utilization =	<u>36%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.673 k
Allowable Uplift =	4.357 k
Utilization =	<u>38%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.978 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>40%</u>

#### Rear Strut

Maximum Axial Load =	3.121 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>42%</u>

#### Diagonal Strut

Maximum Axial Load =	1.638 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>22%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	46.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.938 in
	<u>0.674 ≤ 0.938, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 129 \text{ in}$$

$$J = 0.432$$

$$356.874$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 129$$

$$J = 0.432$$

$$226.951$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \text{ in} \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.46712$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7854$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 12.7711 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 12.77 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 13.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.9	-46.9	0	0
2	M14	Y	-46.9	-46.9	0	0
3	M15	Y	-46.9	-46.9	0	0
4	M16	Y	-46.9	-46.9	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-58.278	-58.278	0	0
2	M14	y	-58.278	-58.278	0	0
3	M15	y	-90.067	-90.067	0	0
4	M16	y	-90.067	-90.067	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	132.451	132.451	0	0
2	M14	y	100.663	100.663	0	0
3	M15	y	52.98	52.98	0	0
4	M16	y	52.98	52.98	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



## RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 25° 110mph 30psf 10.75ft 7-10.r3d]Page 19



Company : Schletter, Inc.  
Designer : HCV  
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Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	126.589	1	857.044	3	204.571	1	.011	1	.38	1	1.6	1
20		min	6.06	12	-728.706	1	-120.987	14	0	12	.013	12	-1.841	3
21	11	max	126.589	1	599.846	1	-6.205	12	.011	1	.162	1	.807	1
22		min	6.06	12	-704.64	3	-160.712	1	0	3	.005	12	-.908	3
23	12	max	126.589	1	470.986	1	-4.402	12	.011	1	.058	4	.167	1
24		min	6.06	12	-552.235	3	-116.854	1	0	3	-.004	1	-.157	3
25	13	max	126.589	1	342.126	1	-2.598	12	.011	1	.026	5	.411	3
26		min	6.06	12	-399.831	3	-72.995	1	0	3	-.117	1	-.319	1
27	14	max	126.589	1	213.266	1	-.795	12	.011	1	-.002	15	.798	3
28		min	6.06	12	-247.427	3	-29.281	4	0	3	-.178	1	-.65	1
29	15	max	126.589	1	84.406	1	14.722	1	.011	1	-.008	12	1.002	3
30		min	-2.178	5	-95.022	3	-19.721	5	0	3	-.187	1	-.828	1
31	16	max	126.589	1	57.382	3	58.581	1	.011	1	-.005	12	1.025	3
32		min	-14.439	5	-44.454	1	-16.93	5	0	3	-.143	1	-.852	1
33	17	max	126.589	1	209.787	3	102.439	1	.011	1	0	12	.865	3
34		min	-26.7	5	-173.314	1	-14.14	5	0	3	-.08	4	-.722	1
35	18	max	126.589	1	362.191	3	146.298	1	.011	1	.102	1	.524	3
36		min	-38.961	5	-302.174	1	-11.35	5	0	3	-.084	5	-.438	1
37	19	max	126.589	1	514.595	3	190.156	1	.011	1	.303	1	0	1
38		min	-51.223	5	-431.034	1	-8.56	5	0	3	-.096	5	0	3
39	M14	1	max	64.792	4	451.801	1	-8.443	12	.006	.342	1	0	1
40		min	2.554	12	-395.361	3	-195.707	1	-.009	1	.016	12	0	3
41	2	max	56.253	1	322.941	1	-6.639	12	.006	3	.17	4	.404	3
42		min	2.554	12	-280.926	3	-151.848	1	-.009	1	.007	12	-.463	1
43	3	max	56.253	1	194.081	1	-4.836	12	.006	3	.091	5	.671	3
44		min	2.554	12	-166.491	3	-107.99	1	-.009	1	-.021	1	-.771	1
45	4	max	56.253	1	65.221	1	-3.033	12	.006	3	.048	5	.802	3
46		min	2.554	12	-52.055	3	-64.131	1	-.009	1	-.123	1	-.926	1
47	5	max	56.253	1	62.38	3	-1.229	12	.006	3	.008	5	.795	3
48		min	2.203	15	-63.639	1	-37.077	4	-.009	1	-.174	1	-.927	1
49	6	max	56.253	1	176.815	3	23.586	1	.006	3	-.007	12	.653	3
50		min	-8.92	5	-192.499	1	-29.126	5	-.009	1	-.172	1	-.774	1
51	7	max	56.253	1	291.251	3	67.445	1	.006	3	-.006	12	.373	3
52		min	-21.181	5	-321.359	1	-26.336	5	-.009	1	-.117	1	-.467	1
53	8	max	56.253	1	405.686	3	111.303	1	.006	3	0	10	0	9
54		min	-33.442	5	-450.219	1	-23.546	5	-.009	1	-.095	4	-.043	3
55	9	max	56.253	1	520.121	3	155.162	1	.006	3	.149	1	.608	1
56		min	-45.704	5	-579.079	1	-20.756	5	-.009	1	-.117	5	-.596	3
57	10	max	77.119	4	634.557	3	199.021	1	.009	1	.36	1	1.377	1
58		min	2.554	12	-707.939	1	-123.255	14	-.006	3	.013	12	-1.286	3
59	11	max	64.858	4	579.079	1	-5.984	12	.009	1	.171	4	.608	1
60		min	2.554	12	-520.121	3	-155.162	1	-.006	3	.004	12	-.596	3
61	12	max	56.253	1	450.219	1	-4.181	12	.009	1	.089	5	0	9
62		min	2.554	12	-405.686	3	-111.303	1	-.006	3	-.011	1	-.043	3
63	13	max	56.253	1	321.359	1	-2.377	12	.009	1	.046	5	.373	3
64		min	2.554	12	-291.251	3	-67.445	1	-.006	3	-.117	1	-.467	1
65	14	max	56.253	1	192.499	1	-.574	12	.009	1	.006	5	.653	3
66		min	2.554	12	-176.815	3	-37.875	4	-.006	3	-.172	1	-.774	1
67	15	max	56.253	1	63.639	1	20.273	1	.009	1	-.007	12	.795	3
68		min	2.257	15	-62.38	3	-29.306	5	-.006	3	-.174	1	-.927	1
69	16	max	56.253	1	52.055	3	64.131	1	.009	1	-.004	12	.802	3
70		min	-8.837	5	-65.221	1	-26.516	5	-.006	3	-.123	1	-.926	1
71	17	max	56.253	1	166.491	3	107.99	1	.009	1	0	3	.671	3
72		min	-21.098	5	-194.081	1	-23.726	5	-.006	3	-.1	4	-.771	1
73	18	max	56.253	1	280.926	3	151.848	1	.009	1	.135	1	.404	3
74		min	-33.359	5	-322.941	1	-20.936	5	-.006	3	-.121	5	-.463	1
75	19	max	56.253	1	395.361	3	195.707	1	.009	1	.342	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-45.621	5	-451.801	1	-18.145	5	-.006	3	-.144	5	0	3
77	M15	1	max	88.74	5	515.848	1	-8.411	12	.009	1	.342	1	0	2
78			min	-59.364	1	-203.274	3	-195.673	1	-.005	3	.016	12	0	12
79		2	max	76.479	5	368.003	1	-6.608	12	.009	1	.208	4	.208	3
80			min	-59.364	1	-145.793	3	-151.814	1	-.005	3	.007	12	-.528	1
81		3	max	64.218	5	220.158	1	-4.805	12	.009	1	.118	5	.348	3
82			min	-59.364	1	-88.313	3	-107.955	1	-.005	3	-.021	1	-.879	1
83		4	max	51.956	5	72.312	1	-3.001	12	.009	1	.065	5	.419	3
84			min	-59.364	1	-30.832	3	-64.097	1	-.005	3	-.123	1	-1.054	1
85		5	max	39.695	5	26.649	3	-1.198	12	.009	1	.014	5	.422	3
86			min	-59.364	1	-75.769	2	-45.877	4	-.005	3	-.174	1	-1.052	1
87		6	max	27.434	5	84.129	3	23.62	1	.009	1	-.007	12	.356	3
88			min	-59.364	1	-223.378	1	-37.899	5	-.005	3	-.172	1	-.873	1
89		7	max	15.173	5	141.61	3	67.479	1	.009	1	-.006	12	.221	3
90			min	-59.364	1	-371.223	1	-35.109	5	-.005	3	-.117	1	-.518	1
91		8	max	2.912	5	199.091	3	111.338	1	.009	1	0	10	.024	2
92			min	-59.364	1	-519.068	1	-32.319	5	-.005	3	-.121	4	0	15
93		9	max	-2.895	12	256.571	3	155.196	1	.009	1	.149	1	.724	2
94			min	-59.364	1	-666.913	1	-29.529	5	-.005	3	-.153	5	-.255	3
95		10	max	-2.895	12	314.052	3	199.055	1	.005	3	.36	1	1.607	1
96			min	-59.364	1	-814.758	1	-127.749	14	-.009	1	.013	12	-.595	3
97		11	max	1.39	5	666.913	1	-6.015	12	.005	3	.207	4	.724	2
98			min	-59.364	1	-256.571	3	-155.196	1	-.009	1	.005	12	-.255	3
99		12	max	-2.895	12	519.068	1	-4.212	12	.005	3	.115	5	.024	2
100			min	-59.364	1	-199.091	3	-111.338	1	-.009	1	-.011	1	0	15
101		13	max	-2.895	12	371.223	1	-2.409	12	.005	3	.061	5	.221	3
102			min	-59.364	1	-141.61	3	-67.479	1	-.009	1	-.117	1	-.518	1
103		14	max	-2.895	12	223.378	1	-.605	12	.005	3	.011	5	.356	3
104			min	-59.364	1	-84.129	3	-46.7	4	-.009	1	-.172	1	-.873	1
105		15	max	-2.895	12	75.769	2	20.238	1	.005	3	-.007	12	.422	3
106			min	-61.419	4	-26.649	3	-38.083	5	-.009	1	-.174	1	-1.052	1
107		16	max	-2.895	12	30.832	3	64.097	1	.005	3	-.004	12	.419	3
108			min	-73.68	4	-72.312	1	-35.293	5	-.009	1	-.123	1	-1.054	1
109		17	max	-2.895	12	88.313	3	107.955	1	.005	3	0	3	.348	3
110			min	-85.941	4	-220.158	1	-32.502	5	-.009	1	-.127	4	-.879	1
111		18	max	-2.895	12	145.793	3	151.814	1	.005	3	.134	1	.208	3
112			min	-98.203	4	-368.003	1	-29.712	5	-.009	1	-.158	5	-.528	1
113		19	max	-2.895	12	203.274	3	195.673	1	.005	3	.342	1	0	2
114			min	-110.464	4	-515.848	1	-26.922	5	-.009	1	-.192	5	0	5
115	M16	1	max	87.04	5	495.206	1	-8.122	12	.01	1	.304	1	0	2
116			min	-135.014	1	-191.398	3	-190.375	1	-.007	3	.014	12	0	3
117		2	max	74.779	5	347.361	1	-6.319	12	.01	1	.158	4	.194	3
118			min	-135.014	1	-133.917	3	-146.517	1	-.007	3	.005	12	-.503	1
119		3	max	62.517	5	199.516	1	-4.516	12	.01	1	.089	5	.32	3
120			min	-135.014	1	-76.437	3	-102.658	1	-.007	3	-.046	1	-.83	1
121		4	max	50.256	5	51.671	1	-2.712	12	.01	1	.048	5	.377	3
122			min	-135.014	1	-18.956	3	-58.799	1	-.007	3	-.143	1	-.98	1
123		5	max	37.995	5	38.525	3	-.909	12	.01	1	.011	5	.365	3
124			min	-135.014	1	-96.174	1	-33.903	4	-.007	3	-.187	1	-.953	1
125		6	max	25.734	5	96.005	3	28.918	1	.01	1	-.008	12	.285	3
126			min	-135.014	1	-244.019	1	-27.275	5	-.007	3	-.178	1	-.75	1
127		7	max	13.473	5	153.486	3	72.777	1	.01	1	-.005	12	.136	3
128			min	-135.014	1	-391.864	1	-24.485	5	-.007	3	-.117	1	-.37	1
129		8	max	1.211	5	210.967	3	116.635	1	.01	1	0	10	.186	1
130			min	-135.014	1	-539.709	1	-21.695	5	-.007	3	-.085	4	-.082	3
131		9	max	-6.263	12	268.447	3	160.494	1	.01	1	.161	1	.919	1
132			min	-135.014	1	-687.554	1	-18.905	5	-.007	3	-.106	5	-.368	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-6.263	12	325.928	3	204.352	1	.007	3	.379	1	1.829	1
134		min	-135.014	1	-835.4	1	-125.24	14	-.01	1	.014	12	-.723	3
135	11	max	-3.861	15	687.554	1	-6.305	12	.007	3	.164	4	.919	1
136		min	-135.014	1	-268.447	3	-160.494	1	-.01	1	.005	12	-.368	3
137	12	max	-6.263	12	539.709	1	-4.501	12	.007	3	.082	4	.186	1
138		min	-135.014	1	-210.967	3	-116.635	1	-.01	1	-.004	1	-.082	3
139	13	max	-6.263	12	391.864	1	-2.698	12	.007	3	.039	5	.136	3
140		min	-135.014	1	-153.486	3	-72.777	1	-.01	1	-.117	1	-.37	1
141	14	max	-6.263	12	244.019	1	-.894	12	.007	3	0	15	.285	3
142		min	-135.014	1	-96.005	3	-37.751	4	-.01	1	-.178	1	-.75	1
143	15	max	-6.263	12	96.174	1	14.941	1	.007	3	-.008	12	.365	3
144		min	-135.014	1	-38.525	3	-28.146	5	-.01	1	-.187	1	-.953	1
145	16	max	-6.263	12	18.956	3	58.799	1	.007	3	-.005	12	.377	3
146		min	-135.014	1	-51.671	1	-25.356	5	-.01	1	-.143	1	-.98	1
147	17	max	-6.263	12	76.437	3	102.658	1	.007	3	-.001	12	.32	3
148		min	-135.014	1	-199.516	1	-22.565	5	-.01	1	-.107	4	-.83	1
149	18	max	-6.263	12	133.917	3	146.517	1	.007	3	.103	1	.194	3
150		min	-135.014	1	-347.361	1	-19.775	5	-.01	1	-.12	5	-.503	1
151	19	max	-6.263	12	191.398	3	190.375	1	.007	3	.304	1	0	2
152		min	-135.014	1	-495.206	1	-16.985	5	-.01	1	-.142	5	0	5
153	M2	1	max	963.712	1	1.956	4	.814	1	0	12	0	3	0
154		min	-924.625	3	.476	15	-48.921	4	0	4	0	1	0	1
155	2	max	964.141	1	1.9	4	.814	1	0	12	0	1	0	15
156		min	-924.303	3	.463	15	-49.294	4	0	4	-.014	4	0	4
157	3	max	964.569	1	1.843	4	.814	1	0	12	0	1	0	15
158		min	-923.982	3	.449	15	-49.667	4	0	4	-.029	4	-.001	4
159	4	max	964.998	1	1.786	4	.814	1	0	12	0	1	0	15
160		min	-923.661	3	.436	15	-50.041	4	0	4	-.043	4	-.002	4
161	5	max	965.426	1	1.729	4	.814	1	0	12	0	1	0	15
162		min	-923.339	3	.423	15	-50.414	4	0	4	-.058	4	-.002	4
163	6	max	965.855	1	1.672	4	.814	1	0	12	.001	1	0	15
164		min	-923.018	3	.409	15	-50.787	4	0	4	-.072	4	-.003	4
165	7	max	966.283	1	1.616	4	.814	1	0	12	.001	1	0	15
166		min	-922.697	3	.396	15	-51.161	4	0	4	-.087	4	-.003	4
167	8	max	966.712	1	1.559	4	.814	1	0	12	.002	1	0	15
168		min	-922.375	3	.383	15	-51.534	4	0	4	-.102	4	-.004	4
169	9	max	967.14	1	1.502	4	.814	1	0	12	.002	1	0	15
170		min	-922.054	3	.369	15	-51.907	4	0	4	-.117	4	-.004	4
171	10	max	967.569	1	1.445	4	.814	1	0	12	.002	1	-.001	15
172		min	-921.732	3	.356	15	-52.281	4	0	4	-.132	4	-.004	4
173	11	max	967.997	1	1.388	4	.814	1	0	12	.002	1	-.001	15
174		min	-921.411	3	.343	15	-52.654	4	0	4	-.147	4	-.005	4
175	12	max	968.426	1	1.332	4	.814	1	0	12	.003	1	-.001	15
176		min	-921.09	3	.329	15	-53.027	4	0	4	-.163	4	-.005	4
177	13	max	968.854	1	1.275	4	.814	1	0	12	.003	1	-.001	15
178		min	-920.768	3	.316	15	-53.401	4	0	4	-.178	4	-.006	4
179	14	max	969.283	1	1.218	4	.814	1	0	12	.003	1	-.001	15
180		min	-920.447	3	.303	15	-53.774	4	0	4	-.194	4	-.006	4
181	15	max	969.711	1	1.161	4	.814	1	0	12	.003	1	-.002	15
182		min	-920.126	3	.289	15	-54.147	4	0	4	-.209	4	-.006	4
183	16	max	970.14	1	1.105	4	.814	1	0	12	.004	1	-.002	15
184		min	-919.804	3	.274	12	-54.52	4	0	4	-.225	4	-.007	4
185	17	max	970.568	1	1.048	4	.814	1	0	12	.004	1	-.002	15
186		min	-919.483	3	.252	12	-54.894	4	0	4	-.241	4	-.007	4
187	18	max	970.997	1	.991	4	.814	1	0	12	.004	1	-.002	15
188		min	-919.162	3	.23	12	-55.267	4	0	4	-.257	4	-.007	4
189	19	max	971.425	1	.934	4	.814	1	0	12	.004	1	-.002	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-918.84	3	.208	12	-55.64	4	0	4	-.273	4	-.008	4
191	M3	1	max	380.433	2	7.906	4	3.734	4	0	12	0	.008	4
192		min	-521.205	3	1.87	15	.008	12	0	4	-.029	4	.002	15
193		2	max	380.263	2	7.139	4	4.273	4	0	12	0	.004	4
194		min	-521.333	3	1.689	15	.008	12	0	4	-.028	4	0	12
195		3	max	380.093	2	6.372	4	4.812	4	0	12	0	.002	2
196		min	-521.461	3	1.509	15	.008	12	0	4	-.026	4	0	3
197		4	max	379.922	2	5.605	4	5.351	4	0	12	0	0	2
198		min	-521.588	3	1.329	15	.008	12	0	4	-.023	4	-.002	3
199		5	max	379.752	2	4.838	4	5.889	4	0	12	0	0	15
200		min	-521.716	3	1.148	15	.008	12	0	4	-.021	4	-.003	6
201		6	max	379.582	2	4.07	4	6.428	4	0	12	0	1	15
202		min	-521.844	3	.968	15	.008	12	0	4	-.019	4	-.005	6
203		7	max	379.411	2	3.303	4	6.967	4	0	12	0	1	15
204		min	-521.972	3	.788	15	.008	12	0	4	-.016	4	-.007	6
205		8	max	379.241	2	2.536	4	7.506	4	0	12	0	1	15
206		min	-522.099	3	.607	15	.008	12	0	4	-.013	5	-.008	6
207		9	max	379.071	2	1.769	4	8.044	4	0	12	.001	1	15
208		min	-522.227	3	.427	15	.008	12	0	4	-.01	5	-.009	6
209		10	max	378.9	2	1.001	4	8.583	4	0	12	.001	1	15
210		min	-522.355	3	.247	15	.008	12	0	4	-.006	5	-.009	6
211		11	max	378.73	2	.283	2	9.122	4	0	12	.001	1	15
212		min	-522.483	3	-.042	3	.008	12	0	4	-.003	5	-.01	6
213		12	max	378.559	2	-.114	15	9.661	4	0	12	.002	4	15
214		min	-522.61	3	-.534	6	.008	12	0	4	0	12	-.009	6
215		13	max	378.389	2	-.294	15	10.199	4	0	12	.006	4	15
216		min	-522.738	3	-1.301	6	.008	12	0	4	0	12	-.009	6
217		14	max	378.219	2	-.475	15	10.738	4	0	12	.01	4	15
218		min	-522.866	3	-2.068	6	.008	12	0	4	0	12	-.008	6
219		15	max	378.048	2	-.655	15	11.277	4	0	12	.015	4	15
220		min	-522.994	3	-2.836	6	.008	12	0	4	0	12	-.007	6
221		16	max	377.878	2	-.835	15	11.816	4	0	12	.02	4	15
222		min	-523.121	3	-3.603	6	.008	12	0	4	0	12	-.006	6
223		17	max	377.708	2	-1.016	15	12.354	4	0	12	.025	4	15
224		min	-523.249	3	-4.37	6	.008	12	0	4	0	12	-.004	6
225		18	max	377.537	2	-1.196	15	12.893	4	0	12	.03	4	15
226		min	-523.377	3	-5.137	6	.008	12	0	4	0	12	-.002	6
227		19	max	377.367	2	-1.376	15	13.432	4	0	12	.036	4	1
228		min	-523.505	3	-5.904	6	.008	12	0	4	0	12	0	1
229	M4	1	max	1114.703	1	0	1	-.589	12	0	1	.026	4	1
230		min	-75.155	3	0	1	-293.345	4	0	1	0	12	0	1
231		2	max	1114.873	1	0	1	-.589	12	0	1	0	12	1
232		min	-75.027	3	0	1	-293.493	4	0	1	-.008	4	0	1
233		3	max	1115.043	1	0	1	-.589	12	0	1	0	12	1
234		min	-74.899	3	0	1	-293.64	4	0	1	-.042	4	0	1
235		4	max	1115.214	1	0	1	-.589	12	0	1	0	12	1
236		min	-74.771	3	0	1	-293.788	4	0	1	-.075	4	0	1
237		5	max	1115.384	1	0	1	-.589	12	0	1	0	12	1
238		min	-74.644	3	0	1	-293.936	4	0	1	-.109	4	0	1
239		6	max	1115.554	1	0	1	-.589	12	0	1	0	12	1
240		min	-74.516	3	0	1	-294.083	4	0	1	-.143	4	0	1
241		7	max	1115.725	1	0	1	-.589	12	0	1	0	12	1
242		min	-74.388	3	0	1	-294.231	4	0	1	-.177	4	0	1
243		8	max	1115.895	1	0	1	-.589	12	0	1	0	12	1
244		min	-74.26	3	0	1	-294.379	4	0	1	-.211	4	0	1
245		9	max	1116.065	1	0	1	-.589	12	0	1	0	12	1
246		min	-74.133	3	0	1	-294.526	4	0	1	-.244	4	0	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1116.236	1	0	1	-589	12	0	1	0	12	0	1
248			min	-74.005	3	0	1	-294.674	4	0	1	-.278	4	0	1
249		11	max	1116.406	1	0	1	-589	12	0	1	0	12	0	1
250			min	-73.877	3	0	1	-294.821	4	0	1	-.312	4	0	1
251		12	max	1116.576	1	0	1	-589	12	0	1	0	12	0	1
252			min	-73.749	3	0	1	-294.969	4	0	1	-.346	4	0	1
253		13	max	1116.747	1	0	1	-589	12	0	1	0	12	0	1
254			min	-73.622	3	0	1	-295.117	4	0	1	-.38	4	0	1
255		14	max	1116.917	1	0	1	-589	12	0	1	0	12	0	1
256			min	-73.494	3	0	1	-295.264	4	0	1	-.414	4	0	1
257		15	max	1117.088	1	0	1	-589	12	0	1	0	12	0	1
258			min	-73.366	3	0	1	-295.412	4	0	1	-.448	4	0	1
259		16	max	1117.258	1	0	1	-589	12	0	1	0	12	0	1
260			min	-73.238	3	0	1	-295.56	4	0	1	-.481	4	0	1
261		17	max	1117.428	1	0	1	-589	12	0	1	-.001	12	0	1
262			min	-73.111	3	0	1	-295.707	4	0	1	-.515	4	0	1
263		18	max	1117.599	1	0	1	-589	12	0	1	-.001	12	0	1
264			min	-72.983	3	0	1	-295.855	4	0	1	-.549	4	0	1
265		19	max	1117.769	1	0	1	-589	12	0	1	-.001	12	0	1
266			min	-72.855	3	0	1	-296.002	4	0	1	-.583	4	0	1
267	M6	1	max	3113.77	1	2.152	2	0	1	0	1	0	4	0	1
268			min	-3042.534	3	.289	12	-49.43	4	0	4	0	1	0	1
269		2	max	3114.198	1	2.107	2	0	1	0	1	0	1	0	12
270			min	-3042.213	3	.267	12	-49.803	4	0	4	-.014	4	0	2
271		3	max	3114.627	1	2.063	2	0	1	0	1	0	1	0	12
272			min	-3041.891	3	.245	12	-50.176	4	0	4	-.029	4	-.001	2
273		4	max	3115.055	1	2.019	2	0	1	0	1	0	1	0	12
274			min	-3041.57	3	.223	12	-50.55	4	0	4	-.044	4	-.002	2
275		5	max	3115.484	1	1.975	2	0	1	0	1	0	1	0	12
276			min	-3041.248	3	.201	12	-50.923	4	0	4	-.058	4	-.002	2
277		6	max	3115.912	1	1.93	2	0	1	0	1	0	1	0	12
278			min	-3040.927	3	.179	12	-51.296	4	0	4	-.073	4	-.003	2
279		7	max	3116.341	1	1.886	2	0	1	0	1	0	1	0	12
280			min	-3040.606	3	.157	12	-51.67	4	0	4	-.088	4	-.004	2
281		8	max	3116.769	1	1.842	2	0	1	0	1	0	1	0	12
282			min	-3040.284	3	.124	3	-52.043	4	0	4	-.103	4	-.004	2
283		9	max	3117.198	1	1.798	2	0	1	0	1	0	1	0	12
284			min	-3039.963	3	.091	3	-52.416	4	0	4	-.118	4	-.005	2
285		10	max	3117.626	1	1.753	2	0	1	0	1	0	1	0	12
286			min	-3039.642	3	.058	3	-52.79	4	0	4	-.134	4	-.005	2
287		11	max	3118.054	1	1.709	2	0	1	0	1	0	1	0	12
288			min	-3039.32	3	.024	3	-53.163	4	0	4	-.149	4	-.006	2
289		12	max	3118.483	1	1.665	2	0	1	0	1	0	1	0	12
290			min	-3038.999	3	-.009	3	-53.536	4	0	4	-.164	4	-.006	2
291		13	max	3118.911	1	1.621	2	0	1	0	1	0	1	0	12
292			min	-3038.678	3	-.042	3	-53.91	4	0	4	-.18	4	-.007	2
293		14	max	3119.34	1	1.576	2	0	1	0	1	0	1	0	3
294			min	-3038.356	3	-.075	3	-54.283	4	0	4	-.196	4	-.007	2
295		15	max	3119.768	1	1.532	2	0	1	0	1	0	1	0	3
296			min	-3038.035	3	-.108	3	-54.656	4	0	4	-.212	4	-.007	2
297		16	max	3120.197	1	1.488	2	0	1	0	1	0	1	0	3
298			min	-3037.713	3	-.142	3	-55.03	4	0	4	-.227	4	-.008	2
299		17	max	3120.625	1	1.444	2	0	1	0	1	0	1	0	3
300			min	-3037.392	3	-.175	3	-55.403	4	0	4	-.244	4	-.008	2
301		18	max	3121.054	1	1.399	2	0	1	0	1	0	1	0	3
302			min	-3037.071	3	-.208	3	-55.776	4	0	4	-.26	4	-.009	2
303		19	max	3121.482	1	1.355	2	0	1	0	1	0	1	0	3



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Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-3036.749	3	-.241	3	-56.15	4	0	4	-.276	4	-.009	2
305	M7	1	max	1582.574	2	7.92	6	3.51	4	0	1	0	1	.009	2
306			min	-1635.399	3	1.859	15	0	1	0	4	-.029	4	0	3
307		2	max	1582.404	2	7.153	6	4.049	4	0	1	0	1	.007	2
308			min	-1635.526	3	1.679	15	0	1	0	4	-.028	4	-.001	3
309		3	max	1582.234	2	6.386	6	4.588	4	0	1	0	1	.004	2
310			min	-1635.654	3	1.498	15	0	1	0	4	-.026	4	-.003	3
311		4	max	1582.063	2	5.618	6	5.127	4	0	1	0	1	.002	2
312			min	-1635.782	3	1.318	15	0	1	0	4	-.024	4	-.004	3
313		5	max	1581.893	2	4.851	6	5.665	4	0	1	0	1	0	2
314			min	-1635.91	3	1.137	15	0	1	0	4	-.022	4	-.005	3
315		6	max	1581.723	2	4.084	6	6.204	4	0	1	0	1	-.001	15
316			min	-1636.037	3	.957	15	0	1	0	4	-.019	4	-.006	3
317		7	max	1581.552	2	3.317	6	6.743	4	0	1	0	1	-.002	15
318			min	-1636.165	3	.777	15	0	1	0	4	-.017	4	-.007	3
319		8	max	1581.382	2	2.55	6	7.282	4	0	1	0	1	-.002	15
320			min	-1636.293	3	.596	15	0	1	0	4	-.014	4	-.008	4
321		9	max	1581.211	2	1.813	2	7.82	4	0	1	0	1	-.002	15
322			min	-1636.421	3	.362	12	0	1	0	4	-.01	4	-.009	4
323		10	max	1581.041	2	1.215	2	8.359	4	0	1	0	1	-.002	15
324			min	-1636.548	3	.04	3	0	1	0	4	-.007	4	-.009	4
325		11	max	1580.871	2	.617	2	8.898	4	0	1	0	1	-.002	15
326			min	-1636.676	3	-.408	3	0	1	0	4	-.003	4	-.009	4
327		12	max	1580.7	2	.019	2	9.437	4	0	1	0	4	-.002	15
328			min	-1636.804	3	-.857	3	0	1	0	4	0	1	-.009	4
329		13	max	1580.53	2	-.305	15	9.975	4	0	1	.004	4	-.002	15
330			min	-1636.932	3	-1.305	3	0	1	0	4	0	1	-.009	4
331		14	max	1580.36	2	-.486	15	10.514	4	0	1	.009	4	-.002	15
332			min	-1637.059	3	-2.054	4	0	1	0	4	0	1	-.008	4
333		15	max	1580.189	2	-.666	15	11.053	4	0	1	.013	4	-.002	15
334			min	-1637.187	3	-2.821	4	0	1	0	4	0	1	-.007	4
335		16	max	1580.019	2	-.846	15	11.592	4	0	1	.018	4	-.001	15
336			min	-1637.315	3	-3.588	4	0	1	0	4	0	1	-.006	4
337		17	max	1579.849	2	-1.027	15	12.13	4	0	1	.023	4	-.001	15
338			min	-1637.443	3	-4.355	4	0	1	0	4	0	1	-.004	4
339		18	max	1579.678	2	-1.207	15	12.669	4	0	1	.028	4	0	15
340			min	-1637.57	3	-5.123	4	0	1	0	4	0	1	-.002	4
341		19	max	1579.508	2	-1.387	15	13.208	4	0	1	.034	4	0	1
342			min	-1637.698	3	-5.89	4	0	1	0	4	0	1	0	1
343	M8	1	max	2975.172	1	0	1	0	1	0	1	.024	4	0	1
344			min	-325.469	3	0	1	-284.046	4	0	1	0	1	0	1
345		2	max	2975.342	1	0	1	0	1	0	1	0	1	0	1
346			min	-325.342	3	0	1	-284.194	4	0	1	-.008	4	0	1
347		3	max	2975.513	1	0	1	0	1	0	1	0	1	0	1
348			min	-325.214	3	0	1	-284.341	4	0	1	-.041	4	0	1
349		4	max	2975.683	1	0	1	0	1	0	1	0	1	0	1
350			min	-325.086	3	0	1	-284.489	4	0	1	-.074	4	0	1
351		5	max	2975.853	1	0	1	0	1	0	1	0	1	0	1
352			min	-324.958	3	0	1	-284.637	4	0	1	-.106	4	0	1
353		6	max	2976.024	1	0	1	0	1	0	1	0	1	0	1
354			min	-324.831	3	0	1	-284.784	4	0	1	-.139	4	0	1
355		7	max	2976.194	1	0	1	0	1	0	1	0	1	0	1
356			min	-324.703	3	0	1	-284.932	4	0	1	-.172	4	0	1
357		8	max	2976.364	1	0	1	0	1	0	1	0	1	0	1
358			min	-324.575	3	0	1	-285.08	4	0	1	-.204	4	0	1
359		9	max	2976.535	1	0	1	0	1	0	1	0	1	0	1
360			min	-324.447	3	0	1	-285.227	4	0	1	-.237	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2976.705	1	0	1	0	1	0	1	0	1	0	1
362			min	-324.32	3	0	1	-285.375	4	0	1	-.27	4	0	1
363		11	max	2976.875	1	0	1	0	1	0	1	0	1	0	1
364			min	-324.192	3	0	1	-285.522	4	0	1	-.303	4	0	1
365		12	max	2977.046	1	0	1	0	1	0	1	0	1	0	1
366			min	-324.064	3	0	1	-285.67	4	0	1	-.336	4	0	1
367		13	max	2977.216	1	0	1	0	1	0	1	0	1	0	1
368			min	-323.936	3	0	1	-285.818	4	0	1	-.368	4	0	1
369		14	max	2977.386	1	0	1	0	1	0	1	0	1	0	1
370			min	-323.809	3	0	1	-285.965	4	0	1	-.401	4	0	1
371		15	max	2977.557	1	0	1	0	1	0	1	0	1	0	1
372			min	-323.681	3	0	1	-286.113	4	0	1	-.434	4	0	1
373		16	max	2977.727	1	0	1	0	1	0	1	0	1	0	1
374			min	-323.553	3	0	1	-286.261	4	0	1	-.467	4	0	1
375		17	max	2977.897	1	0	1	0	1	0	1	0	1	0	1
376			min	-323.425	3	0	1	-286.408	4	0	1	-.5	4	0	1
377		18	max	2978.068	1	0	1	0	1	0	1	0	1	0	1
378			min	-323.298	3	0	1	-286.556	4	0	1	-.533	4	0	1
379		19	max	2978.238	1	0	1	0	1	0	1	0	1	0	1
380			min	-323.17	3	0	1	-286.704	4	0	1	-.566	4	0	1
381	M10	1	max	963.712	1	1.885	6	-.036	12	0	1	0	1	0	1
382			min	-924.625	3	.428	15	-49.383	4	0	5	0	3	0	1
383		2	max	964.141	1	1.828	6	-.036	12	0	1	0	10	0	15
384			min	-924.303	3	.415	15	-49.756	4	0	5	-.014	4	0	6
385		3	max	964.569	1	1.772	6	-.036	12	0	1	0	12	0	15
386			min	-923.982	3	.402	15	-50.129	4	0	5	-.029	4	-.001	6
387		4	max	964.998	1	1.715	6	-.036	12	0	1	0	12	0	15
388			min	-923.661	3	.388	15	-50.502	4	0	5	-.043	4	-.002	6
389		5	max	965.426	1	1.658	6	-.036	12	0	1	0	12	0	15
390			min	-923.339	3	.375	15	-50.876	4	0	5	-.058	4	-.002	6
391		6	max	965.855	1	1.601	6	-.036	12	0	1	0	12	0	15
392			min	-923.018	3	.362	15	-51.249	4	0	5	-.073	4	-.003	6
393		7	max	966.283	1	1.544	6	-.036	12	0	1	0	12	0	15
394			min	-922.697	3	.348	15	-51.622	4	0	5	-.088	4	-.003	6
395		8	max	966.712	1	1.488	6	-.036	12	0	1	0	12	0	15
396			min	-922.375	3	.335	15	-51.996	4	0	5	-.103	4	-.003	6
397		9	max	967.14	1	1.431	6	-.036	12	0	1	0	12	0	15
398			min	-922.054	3	.322	15	-52.369	4	0	5	-.118	4	-.004	6
399		10	max	967.569	1	1.374	6	-.036	12	0	1	0	12	0	15
400			min	-921.732	3	.308	15	-52.742	4	0	5	-.133	4	-.004	6
401		11	max	967.997	1	1.317	6	-.036	12	0	1	0	12	-.001	15
402			min	-921.411	3	.295	15	-53.116	4	0	5	-.149	4	-.005	6
403		12	max	968.426	1	1.261	6	-.036	12	0	1	0	12	-.001	15
404			min	-921.09	3	.282	15	-53.489	4	0	5	-.164	4	-.005	6
405		13	max	968.854	1	1.204	6	-.036	12	0	1	0	12	-.001	15
406			min	-920.768	3	.268	15	-53.862	4	0	5	-.18	4	-.005	6
407		14	max	969.283	1	1.147	6	-.036	12	0	1	0	12	-.001	15
408			min	-920.447	3	.255	15	-54.236	4	0	5	-.196	4	-.006	6
409		15	max	969.711	1	1.09	6	-.036	12	0	1	0	12	-.001	15
410			min	-920.126	3	.241	15	-54.609	4	0	5	-.211	4	-.006	6
411		16	max	970.14	1	1.033	6	-.036	12	0	1	0	12	-.001	15
412			min	-919.804	3	.228	15	-54.982	4	0	5	-.227	4	-.006	6
413		17	max	970.568	1	.977	6	-.036	12	0	1	0	12	-.001	15
414			min	-919.483	3	.215	15	-55.356	4	0	5	-.243	4	-.007	6
415		18	max	970.997	1	.92	6	-.036	12	0	1	0	12	-.002	15
416			min	-919.162	3	.201	15	-55.729	4	0	5	-.259	4	-.007	6
417		19	max	971.425	1	.873	2	-.036	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418		min	-918.84	3	.188	15	-56.102	4	0	5	-.276	4	-.007	6
419	M11	1	max	380.433	2	7.857	6	3.634	4	0	1	0	.007	6
420		min	-521.205	3	1.837	15	-.178	1	0	4	-.029	4	.002	15
421		2	max	380.263	2	7.09	6	4.173	4	0	1	0	.004	2
422		min	-521.333	3	1.656	15	-.178	1	0	4	-.028	4	0	15
423		3	max	380.093	2	6.323	6	4.711	4	0	1	0	.002	2
424		min	-521.461	3	1.476	15	-.178	1	0	4	-.026	4	0	3
425		4	max	379.922	2	5.556	6	5.25	4	0	1	0	12	2
426		min	-521.588	3	1.296	15	-.178	1	0	4	-.024	4	-.002	3
427		5	max	379.752	2	4.788	6	5.789	4	0	1	0	12	15
428		min	-521.716	3	1.115	15	-.178	1	0	4	-.022	4	-.003	4
429		6	max	379.582	2	4.021	6	6.328	4	0	1	0	12	15
430		min	-521.844	3	.935	15	-.178	1	0	4	-.019	4	-.005	4
431		7	max	379.411	2	3.254	6	6.866	4	0	1	0	12	15
432		min	-521.972	3	.755	15	-.178	1	0	4	-.016	4	-.007	4
433		8	max	379.241	2	2.487	6	7.405	4	0	1	0	12	15
434		min	-522.099	3	.574	15	-.178	1	0	4	-.013	4	-.008	4
435		9	max	379.071	2	1.719	6	7.944	4	0	1	0	12	15
436		min	-522.227	3	.394	15	-.178	1	0	4	-.01	4	-.009	4
437		10	max	378.9	2	.952	6	8.483	4	0	1	0	12	15
438		min	-522.355	3	.214	15	-.178	1	0	4	-.007	4	-.009	4
439		11	max	378.73	2	.283	2	9.021	4	0	1	0	12	15
440		min	-522.483	3	-.042	3	-.178	1	0	4	-.003	4	-.01	4
441		12	max	378.559	2	-.147	15	9.56	4	0	1	.001	5	15
442		min	-522.61	3	-.583	4	-.178	1	0	4	-.001	1	-.01	4
443		13	max	378.389	2	-.327	15	10.099	4	0	1	.005	5	15
444		min	-522.738	3	-1.35	4	-.178	1	0	4	-.001	1	-.009	4
445		14	max	378.219	2	-.508	15	10.637	4	0	1	.01	5	15
446		min	-522.866	3	-2.118	4	-.178	1	0	4	-.001	1	-.008	4
447		15	max	378.048	2	-.688	15	11.176	4	0	1	.014	5	15
448		min	-522.994	3	-2.885	4	-.178	1	0	4	-.001	1	-.007	4
449		16	max	377.878	2	-.869	15	11.715	4	0	1	.019	5	15
450		min	-523.121	3	-3.652	4	-.178	1	0	4	-.002	1	-.006	4
451		17	max	377.708	2	-1.049	15	12.254	4	0	1	.024	5	15
452		min	-523.249	3	-4.419	4	-.178	1	0	4	-.002	1	-.004	4
453		18	max	377.537	2	-1.229	15	12.792	4	0	1	.029	4	15
454		min	-523.377	3	-5.187	4	-.178	1	0	4	-.002	1	-.002	4
455		19	max	377.367	2	-1.41	15	13.331	4	0	1	.035	4	1
456		min	-523.505	3	-5.954	4	-.178	1	0	4	-.002	1	0	1
457	M12	1	max	1114.703	1	0	1	12.795	1	0	1	.025	4	1
458		min	-75.155	3	0	1	-286.065	4	0	1	-.001	1	0	1
459		2	max	1114.873	1	0	1	12.795	1	0	1	0	1	1
460		min	-75.027	3	0	1	-286.212	4	0	1	-.008	4	0	1
461		3	max	1115.043	1	0	1	12.795	1	0	1	.002	1	1
462		min	-74.899	3	0	1	-286.36	4	0	1	-.041	4	0	1
463		4	max	1115.214	1	0	1	12.795	1	0	1	.003	1	1
464		min	-74.771	3	0	1	-286.508	4	0	1	-.074	4	0	1
465		5	max	1115.384	1	0	1	12.795	1	0	1	.005	1	1
466		min	-74.644	3	0	1	-286.655	4	0	1	-.107	4	0	1
467		6	max	1115.554	1	0	1	12.795	1	0	1	.006	1	1
468		min	-74.516	3	0	1	-286.803	4	0	1	-.139	4	0	1
469		7	max	1115.725	1	0	1	12.795	1	0	1	.008	1	1
470		min	-74.388	3	0	1	-286.951	4	0	1	-.172	4	0	1
471		8	max	1115.895	1	0	1	12.795	1	0	1	.009	1	1
472		min	-74.26	3	0	1	-287.098	4	0	1	-.205	4	0	1
473		9	max	1116.065	1	0	1	12.795	1	0	1	.01	1	1
474		min	-74.133	3	0	1	-287.246	4	0	1	-.238	4	0	1



Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1116.236	1	0	1	12.795	1	0	1	.012	1	0	1
476			min	-74.005	3	0	1	-287.393	4	0	1	-.271	4	0	1
477		11	max	1116.406	1	0	1	12.795	1	0	1	.013	1	0	1
478			min	-73.877	3	0	1	-287.541	4	0	1	-.304	4	0	1
479		12	max	1116.576	1	0	1	12.795	1	0	1	.015	1	0	1
480			min	-73.749	3	0	1	-287.689	4	0	1	-.337	4	0	1
481		13	max	1116.747	1	0	1	12.795	1	0	1	.016	1	0	1
482			min	-73.622	3	0	1	-287.836	4	0	1	-.37	4	0	1
483		14	max	1116.917	1	0	1	12.795	1	0	1	.018	1	0	1
484			min	-73.494	3	0	1	-287.984	4	0	1	-.403	4	0	1
485		15	max	1117.088	1	0	1	12.795	1	0	1	.019	1	0	1
486			min	-73.366	3	0	1	-288.132	4	0	1	-.437	4	0	1
487		16	max	1117.258	1	0	1	12.795	1	0	1	.021	1	0	1
488			min	-73.238	3	0	1	-288.279	4	0	1	-.47	4	0	1
489		17	max	1117.428	1	0	1	12.795	1	0	1	.022	1	0	1
490			min	-73.111	3	0	1	-288.427	4	0	1	-.503	4	0	1
491		18	max	1117.599	1	0	1	12.795	1	0	1	.024	1	0	1
492			min	-72.983	3	0	1	-288.575	4	0	1	-.536	4	0	1
493		19	max	1117.769	1	0	1	12.795	1	0	1	.025	1	0	1
494			min	-72.855	3	0	1	-288.722	4	0	1	-.569	4	0	1
495	M1	1	max	190.162	1	514.579	3	51.195	5	0	1	.303	1	0	3
496			min	-8.56	5	-429.671	1	-126.421	1	0	3	-.096	5	-.011	1
497		2	max	190.767	1	513.605	3	52.437	5	0	1	.236	1	.216	1
498			min	-8.278	5	-430.969	1	-126.421	1	0	3	-.068	5	-.271	3
499		3	max	316.645	3	476.334	1	5.983	5	0	3	.169	1	.433	1
500			min	-191.284	2	-362.105	3	-125.889	1	0	1	-.041	5	-.531	3
501		4	max	317.099	3	475.036	1	7.224	5	0	3	.103	1	.182	1
502			min	-190.679	2	-363.079	3	-125.889	1	0	1	-.037	5	-.34	3
503		5	max	317.553	3	473.738	1	8.466	5	0	3	.036	1	-.003	15
504			min	-190.074	2	-364.052	3	-125.889	1	0	1	-.033	5	-.148	3
505		6	max	318.008	3	472.439	1	9.707	5	0	3	-.001	12	.044	3
506			min	-189.468	2	-365.026	3	-125.889	1	0	1	-.035	4	-.318	1
507		7	max	318.462	3	471.141	1	10.949	5	0	3	-.005	12	.237	3
508			min	-188.863	2	-366	3	-125.889	1	0	1	-.097	1	-.567	1
509		8	max	318.916	3	469.843	1	12.19	5	0	3	-.008	12	.43	3
510			min	-188.258	2	-366.973	3	-125.889	1	0	1	-.163	1	-.816	1
511		9	max	332.097	3	32.828	2	58.952	5	0	9	.095	1	.504	3
512			min	-108.544	2	.392	15	-182.174	1	0	3	-.14	5	-.929	1
513		10	max	332.551	3	31.529	2	60.193	5	0	9	0	12	.489	3
514			min	-107.938	2	0	5	-182.174	1	0	3	-.109	4	-.939	1
515		11	max	333.005	3	30.231	2	61.435	5	0	9	-.005	12	.475	3
516			min	-107.333	2	-1.605	4	-182.174	1	0	3	-.098	4	-.947	1
517		12	max	346.136	3	230.586	3	159.164	5	0	1	.161	1	.413	3
518			min	-65.819	5	-504.383	1	-122.894	1	0	3	-.215	5	-.836	1
519		13	max	346.59	3	229.612	3	160.405	5	0	1	.096	1	.292	3
520			min	-65.536	5	-505.681	1	-122.894	1	0	3	-.13	5	-.57	1
521		14	max	347.044	3	228.639	3	161.647	5	0	1	.031	1	.171	3
522			min	-65.254	5	-506.979	1	-122.894	1	0	3	-.045	5	-.303	1
523		15	max	347.498	3	227.665	3	162.888	5	0	1	.04	5	.05	3
524			min	-64.971	5	-508.278	1	-122.894	1	0	3	-.034	1	-.035	1
525		16	max	347.952	3	226.691	3	164.13	5	0	1	.126	5	.242	2
526			min	-64.689	5	-509.576	1	-122.894	1	0	3	-.098	1	-.069	3
527		17	max	348.406	3	225.718	3	165.371	5	0	1	.213	5	.503	1
528			min	-64.406	5	-510.874	1	-122.894	1	0	3	-.163	1	-.189	3
529		18	max	16.702	5	497.786	1	-6.263	12	0	5	.198	5	.252	1
530			min	-190.976	1	-190.47	3	-136.344	4	0	2	-.233	1	-.094	3
531		19	max	16.985	5	496.487	1	-6.263	12	0	5	.142	5	.007	3





Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532	M5	min	-190.371	1	-191.444	3	-135.176	1	0	2	-.304	1	-.01	1
533		max	409.132	1	1714.029	3	102.308	5	0	1	0	1	.023	1
534		min	16.017	12	-1449.145	1	0	1	0	4	-.219	4	0	3
535		max	409.737	1	1713.056	3	103.549	5	0	1	0	1	.788	1
536		min	16.32	12	-1450.443	1	0	1	0	4	-.165	4	-.904	3
537		max	1019.76	3	1468.266	1	60.862	4	0	4	0	1	1.518	1
538		min	-703.702	2	-1175.603	3	0	1	0	1	-.111	4	-1.773	3
539		max	1020.214	3	1466.968	1	62.104	4	0	4	0	1	.744	1
540		min	-703.096	2	-1176.576	3	0	1	0	1	-.078	4	-1.153	3
541		max	1020.668	3	1465.67	1	63.345	4	0	4	0	1	.005	9
542	M6	min	-702.491	2	-1177.55	3	0	1	0	1	-.045	4	-.532	3
543		max	1021.122	3	1464.371	1	64.587	4	0	4	0	1	.09	3
544		min	-701.885	2	-1178.524	3	0	1	0	1	-.012	5	-.803	1
545		max	1021.576	3	1463.073	1	65.828	4	0	4	.023	4	.712	3
546		min	-701.28	2	-1179.497	3	0	1	0	1	0	1	-1.575	1
547		max	1022.03	3	1461.775	1	67.07	4	0	4	.058	4	1.335	3
548		min	-700.675	2	-1180.471	3	0	1	0	1	0	1	-2.347	1
549		max	1045.719	3	108.561	2	191.475	4	0	1	0	1	1.54	3
550		min	-537.593	2	.393	15	0	1	0	1	-.201	4	-2.655	1
551		max	1046.173	3	107.263	2	192.716	4	0	1	0	1	1.488	3
552	M7	min	-536.988	2	.002	15	0	1	0	1	-.1	4	-2.686	1
553		max	1046.627	3	105.964	2	193.958	4	0	1	.002	4	1.436	3
554		min	-536.382	2	-1.408	6	0	1	0	1	0	1	-2.716	1
555		max	1070.419	3	739.335	3	229.914	4	0	1	0	1	1.259	3
556		min	-373.31	2	-1578.861	1	0	1	0	4	-.315	4	-2.42	1
557		max	1070.873	3	738.361	3	231.156	4	0	1	0	1	.869	3
558		min	-372.705	2	-1580.159	1	0	1	0	4	-.193	4	-1.587	1
559		max	1071.327	3	737.388	3	232.397	4	0	1	0	1	.48	3
560		min	-372.099	2	-1581.457	1	0	1	0	4	-.071	4	-.753	1
561		max	1071.781	3	736.414	3	233.639	4	0	1	.052	4	.136	2
562	M8	min	-371.494	2	-1582.756	1	0	1	0	4	0	1	-.004	13
563		max	1072.235	3	735.44	3	234.88	4	0	1	.176	4	.928	2
564		min	-370.889	2	-1584.054	1	0	1	0	4	0	1	-.297	3
565		max	1072.689	3	734.467	3	236.121	4	0	1	.3	4	1.754	1
566		min	-370.283	2	-1585.352	1	0	1	0	4	0	1	-.685	3
567		max	-16.518	12	1679.826	1	0	1	0	4	.323	4	.906	1
568		min	-409.318	1	-651.178	3	-30.765	5	0	1	0	1	-.358	3
569		max	-16.215	12	1678.528	1	0	1	0	4	.308	4	.02	1
570		min	-408.713	1	-652.152	3	-29.524	5	0	1	0	1	-.014	3
571		max	190.162	1	514.579	3	126.421	1	0	3	-.015	12	0	3
572	M9	min	8.222	12	-429.671	1	6.06	12	0	4	-.303	1	-.011	1
573		max	190.767	1	513.605	3	126.421	1	0	3	-.011	12	.216	1
574		min	8.524	12	-430.969	1	6.06	12	0	4	-.236	1	-.271	3
575		max	316.645	3	476.334	1	125.889	1	0	1	-.008	12	.433	1
576		min	-191.284	2	-362.105	3	6.023	12	0	3	-.169	1	-.531	3
577		max	317.099	3	475.036	1	125.889	1	0	1	-.005	12	.182	1
578		min	-190.679	2	-363.079	3	6.023	12	0	3	-.103	1	-.34	3
579		max	317.553	3	473.738	1	125.889	1	0	1	-.002	12	-.003	15
580		min	-190.074	2	-364.052	3	6.023	12	0	3	-.046	4	-.148	3
581		max	318.008	3	472.439	1	125.889	1	0	1	.03	1	.044	3
582	M10	min	-189.468	2	-365.026	3	6.023	12	0	3	-.025	5	-.318	1
583		max	318.462	3	471.141	1	125.889	1	0	1	.097	1	.237	3
584		min	-188.863	2	-.366	3	6.023	12	0	3	-.011	5	-.567	1
585		max	318.916	3	469.843	1	125.889	1	0	1	.163	1	.43	3
586		min	-188.258	2	-366.973	3	6.023	12	0	3	.002	15	-.816	1
587		max	332.097	3	32.828	2	182.174	1	0	3	-.004	12	.504	3
588		min	-108.544	2	.399	15	8.574	12	0	9	-.174	4	-.929	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	332.551	3	31.529	2	182.174	1	0	3	.001	1	.489	3
590		min	-107.938	2	.007	15	8.574	12	0	9	-.109	4	-.939	1
591	11	max	333.005	3	30.231	2	182.174	1	0	3	.097	1	.475	3
592		min	-107.333	2	-1.555	6	8.574	12	0	9	-.064	5	-.947	1
593	12	max	346.136	3	230.586	3	204.462	4	0	3	-.007	12	.413	3
594		min	-62.271	10	-504.383	1	5.682	12	0	1	-.274	4	-.836	1
595	13	max	346.59	3	229.612	3	205.704	4	0	3	-.004	12	.292	3
596		min	-61.767	10	-505.681	1	5.682	12	0	1	-.165	4	-.57	1
597	14	max	347.044	3	228.639	3	206.945	4	0	3	-.001	12	.171	3
598		min	-61.262	10	-506.979	1	5.682	12	0	1	-.057	4	-.303	1
599	15	max	347.498	3	227.665	3	208.187	4	0	3	.053	4	.05	3
600		min	-60.758	10	-508.278	1	5.682	12	0	1	.002	12	-.035	1
601	16	max	347.952	3	226.691	3	209.428	4	0	3	.163	4	.242	2
602		min	-60.253	10	-509.576	1	5.682	12	0	1	.005	12	-.069	3
603	17	max	348.406	3	225.718	3	210.67	4	0	3	.274	4	.503	1
604		min	-59.749	10	-510.874	1	5.682	12	0	1	.008	12	-.189	3
605	18	max	-8.425	12	497.786	1	135.176	1	0	2	.282	4	.252	1
606		min	-190.976	1	-190.47	3	-88.476	5	0	3	.011	12	-.094	3
607	19	max	-8.123	12	496.487	1	135.176	1	0	2	.304	1	.007	3
608		min	-190.371	1	-191.444	3	-87.235	5	0	3	.014	12	-.01	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.092	1	.006	3	7.45e-3	1	NC	1	NC	1
2			min	-.701	4	-.008	3	-.002	2	-7.542e-4	3	NC	1	NC	1
3		2	max	.001	1	.295	3	.054	1	8.634e-3	1	NC	5	NC	2
4			min	-.701	4	-.142	1	-.025	5	-7.601e-4	3	851.483	3	5000.31	1
5		3	max	.001	1	.541	3	.13	1	9.818e-3	1	NC	5	NC	3
6			min	-.701	4	-.327	1	-.029	5	-7.66e-4	3	470.428	3	2026.855	1
7		4	max	0	1	.69	3	.196	1	1.1e-2	1	NC	5	NC	3
8			min	-.701	4	-.433	1	-.02	5	-7.719e-4	3	369.821	3	1334.791	1
9		5	max	0	1	.725	3	.23	1	1.219e-2	1	NC	5	NC	3
10			min	-.701	4	-.444	1	-.002	5	-7.778e-4	3	352.205	3	1132.998	1
11		6	max	0	1	.648	3	.223	1	1.337e-2	1	NC	5	NC	3
12			min	-.701	4	-.362	1	.01	15	-7.837e-4	3	393.494	3	1169.49	1
13		7	max	0	1	.483	3	.176	1	1.455e-2	1	NC	5	NC	3
14			min	-.701	4	-.208	1	.014	10	-7.896e-4	3	526.32	3	1483.349	1
15		8	max	0	1	.272	3	.104	1	1.574e-2	1	NC	4	NC	3
16			min	-.701	4	-.02	1	.005	10	-7.955e-4	3	921.797	3	2540.487	1
17		9	max	0	1	.148	1	.034	4	1.692e-2	1	NC	4	NC	2
18			min	-.701	4	.005	15	-.003	10	-8.014e-4	3	2895.158	3	7552.443	4
19		10	max	0	1	.224	1	.018	3	1.811e-2	1	NC	3	NC	1
20			min	-.701	4	-.005	3	-.012	2	-8.073e-4	3	1966.304	1	NC	1
21		11	max	0	12	.148	1	.032	1	1.692e-2	1	NC	4	NC	2
22			min	-.701	4	.005	15	-.02	5	-8.014e-4	3	2895.158	3	8812.784	1
23		12	max	0	12	.272	3	.104	1	1.574e-2	1	NC	4	NC	3
24			min	-.701	4	-.02	1	-.02	5	-7.955e-4	3	921.797	3	2540.487	1
25		13	max	0	12	.483	3	.176	1	1.455e-2	1	NC	5	NC	3
26			min	-.701	4	-.208	1	-.006	5	-7.896e-4	3	526.32	3	1483.349	1
27		14	max	0	12	.648	3	.223	1	1.337e-2	1	NC	5	NC	3
28			min	-.701	4	-.362	1	.009	15	-7.837e-4	3	393.494	3	1169.49	1
29		15	max	0	12	.725	3	.23	1	1.219e-2	1	NC	5	NC	3
30			min	-.701	4	-.444	1	.016	12	-7.778e-4	3	352.205	3	1132.998	1
31		16	max	0	12	.69	3	.196	1	1.1e-2	1	NC	5	NC	3
32			min	-.701	4	-.433	1	.014	12	-7.719e-4	3	369.821	3	1334.791	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.541	3	.13	1	9.818e-3	1	NC	5	NC	3
34		min	-7.01	4	-.327	1	.01	12	-7.66e-4	3	470.428	3	2026.855	1
35	18	max	0	12	.295	3	.054	1	8.634e-3	1	NC	5	NC	2
36		min	-7.01	4	-.142	1	.004	10	-7.601e-4	3	851.483	3	5000.31	1
37	19	max	0	12	.092	1	.006	3	7.45e-3	1	NC	1	NC	1
38		min	-7.01	4	-.008	3	-.002	2	-7.542e-4	3	NC	1	NC	1
39	M14	1	max	0	.148	3	.005	3	4.681e-3	1	NC	1	NC	1
40		min	-.524	4	-.305	1	-.002	2	-2.688e-3	3	NC	1	NC	1
41	2	max	0	1	.425	3	.038	1	5.646e-3	1	NC	5	NC	2
42		min	-.524	4	-.652	1	-.036	5	-3.295e-3	3	742.759	1	7237.42	1
43	3	max	0	1	.657	3	.105	1	6.611e-3	1	NC	15	NC	3
44		min	-.524	4	-.949	1	-.043	5	-3.902e-3	3	400.761	1	2506.804	1
45	4	max	0	1	.814	3	.169	1	7.576e-3	1	NC	15	NC	3
46		min	-.524	4	-1.159	1	-.028	5	-4.508e-3	3	301.999	1	1549.419	1
47	5	max	0	1	.882	3	.205	1	8.541e-3	1	9139.988	15	NC	3
48		min	-.524	4	-1.266	1	-.003	5	-5.115e-3	3	268.354	1	1270.647	1
49	6	max	0	1	.861	3	.203	1	9.507e-3	1	9146.024	15	NC	3
50		min	-.524	4	-1.27	1	.015	12	-5.722e-3	3	267.358	1	1283.268	1
51	7	max	0	1	.768	3	.163	1	1.047e-2	1	NC	15	NC	3
52		min	-.524	4	-1.187	1	.013	10	-6.328e-3	3	292.394	1	1602.75	1
53	8	max	0	1	.633	3	.098	1	1.144e-2	1	NC	15	NC	3
54		min	-.524	4	-1.053	1	.005	10	-6.935e-3	3	344.793	1	2710.484	1
55	9	max	0	1	.505	3	.049	4	1.24e-2	1	NC	15	NC	2
56		min	-.525	4	-.92	1	-.003	10	-7.542e-3	3	419.592	1	5259.11	4
57	10	max	0	1	.446	3	.016	3	1.337e-2	1	NC	5	NC	1
58		min	-.525	4	-.857	1	-.01	2	-8.148e-3	3	467.569	1	NC	1
59	11	max	0	12	.505	3	.03	1	1.24e-2	1	NC	15	NC	2
60		min	-.525	4	-.92	1	-.036	5	-7.542e-3	3	419.592	1	7467.482	5
61	12	max	0	12	.633	3	.098	1	1.144e-2	1	NC	15	NC	3
62		min	-.525	4	-1.053	1	-.04	5	-6.935e-3	3	344.793	1	2710.484	1
63	13	max	0	12	.768	3	.163	1	1.047e-2	1	NC	15	NC	3
64		min	-.525	4	-1.187	1	-.025	5	-6.328e-3	3	292.394	1	1602.75	1
65	14	max	0	12	.861	3	.203	1	9.507e-3	1	9145.668	15	NC	3
66		min	-.525	4	-1.27	1	0	15	-5.722e-3	3	267.358	1	1283.268	1
67	15	max	0	12	.882	3	.205	1	8.541e-3	1	9139.544	15	NC	3
68		min	-.525	4	-1.266	1	.014	12	-5.115e-3	3	268.354	1	1270.647	1
69	16	max	0	12	.814	3	.169	1	7.576e-3	1	NC	15	NC	3
70		min	-.525	4	-1.159	1	.012	12	-4.508e-3	3	301.999	1	1549.419	1
71	17	max	0	12	.657	3	.105	1	6.611e-3	1	NC	15	NC	3
72		min	-.525	4	-.949	1	.008	12	-3.902e-3	3	400.761	1	2506.804	1
73	18	max	0	12	.425	3	.051	4	5.646e-3	1	NC	5	NC	2
74		min	-.525	4	-.652	1	.002	10	-3.295e-3	3	742.759	1	5056.37	4
75	19	max	0	12	.148	3	.005	3	4.681e-3	1	NC	1	NC	1
76		min	-.525	4	-.305	1	-.002	2	-2.688e-3	3	NC	1	NC	1
77	M15	1	max	0	.152	3	.005	3	2.257e-3	3	NC	1	NC	1
78		min	-.427	4	-.304	1	-.002	2	-4.797e-3	1	NC	1	NC	1
79	2	max	0	12	.316	3	.038	1	2.77e-3	3	NC	5	NC	2
80		min	-.427	4	-.689	1	-.048	5	-5.792e-3	1	670.498	1	5229.098	5
81	3	max	0	12	.458	3	.105	1	3.284e-3	3	NC	15	NC	3
82		min	-.427	4	-1.016	1	-.058	5	-6.788e-3	1	362.623	1	2500.206	1
83	4	max	0	12	.561	3	.169	1	3.798e-3	3	NC	15	NC	3
84		min	-.427	4	-1.245	1	-.04	5	-7.784e-3	1	274.376	1	1546.303	1
85	5	max	0	12	.616	3	.206	1	4.312e-3	3	9152.015	15	NC	3
86		min	-.427	4	-1.356	1	-.008	5	-8.78e-3	1	245.362	1	1268.374	1
87	6	max	0	12	.625	3	.204	1	4.826e-3	3	9160.508	15	NC	3
88		min	-.427	4	-1.35	1	.015	12	-9.776e-3	1	246.823	1	1280.934	1
89	7	max	0	12	.594	3	.164	1	5.34e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.427	4	-1.246	1	.014	10	-1.077e-2	1	273.93	1	1599.254	1
91	8	max	0	12	.539	3	.098	1	5.854e-3	3	NC	15	NC	3
92		min	-.427	4	-1.086	1	.005	10	-1.177e-2	1	330.175	1	2701.12	1
93	9	max	0	12	.482	3	.06	4	6.368e-3	3	NC	15	NC	2
94		min	-.427	4	-.928	1	-.003	10	-1.276e-2	1	413.361	1	4331.419	4
95	10	max	0	1	.455	3	.015	3	6.882e-3	3	NC	5	NC	1
96		min	-.427	4	-.855	1	-.01	2	-1.376e-2	1	468.851	1	NC	1
97	11	max	0	1	.482	3	.031	1	6.368e-3	3	NC	15	NC	2
98		min	-.427	4	-.928	1	-.046	5	-1.276e-2	1	413.361	1	5582.162	5
99	12	max	0	1	.539	3	.098	1	5.854e-3	3	NC	15	NC	3
100		min	-.427	4	-1.086	1	-.053	5	-1.177e-2	1	330.175	1	2701.12	1
101	13	max	0	1	.594	3	.164	1	5.34e-3	3	NC	15	NC	3
102		min	-.427	4	-1.246	1	-.034	5	-1.077e-2	1	273.93	1	1599.254	1
103	14	max	0	1	.625	3	.204	1	4.826e-3	3	9160.239	15	NC	3
104		min	-.427	4	-1.35	1	0	15	-9.776e-3	1	246.823	1	1280.934	1
105	15	max	0	1	.616	3	.206	1	4.312e-3	3	9151.681	15	NC	3
106		min	-.427	4	-1.356	1	.014	12	-8.78e-3	1	245.362	1	1268.374	1
107	16	max	0	1	.561	3	.169	1	3.798e-3	3	NC	15	NC	3
108		min	-.427	4	-1.245	1	.011	12	-7.784e-3	1	274.376	1	1546.303	1
109	17	max	0	1	.458	3	.105	1	3.284e-3	3	NC	15	NC	3
110		min	-.427	4	-1.016	1	.008	12	-6.788e-3	1	362.623	1	2500.206	1
111	18	max	0	1	.316	3	.063	4	2.77e-3	3	NC	5	NC	2
112		min	-.427	4	-.689	1	.002	10	-5.792e-3	1	670.498	1	4103.791	4
113	19	max	0	1	.152	3	.005	3	2.257e-3	3	NC	1	NC	1
114		min	-.427	4	-.304	1	-.002	2	-4.797e-3	1	NC	1	NC	1
115	M16	1	max	0	.091	1	.004	3	3.97e-3	3	NC	1	NC	1
116		min	-.153	4	-.05	3	-.002	2	-6.975e-3	1	NC	1	NC	1
117	2	max	0	12	.05	3	.053	1	4.731e-3	3	NC	5	NC	2
118		min	-.153	4	-.193	2	-.037	5	-8.042e-3	1	948.047	1	5035.175	1
119	3	max	0	12	.128	3	.129	1	5.492e-3	3	NC	5	NC	3
120		min	-.153	4	-.41	2	-.045	5	-9.109e-3	1	527.635	1	2034.044	1
121	4	max	0	12	.171	3	.195	1	6.252e-3	3	NC	5	NC	3
122		min	-.153	4	-.536	2	-.033	5	-1.018e-2	1	420.028	2	1337.124	1
123	5	max	0	12	.172	3	.23	1	7.013e-3	3	NC	5	NC	3
124		min	-.153	4	-.554	2	-.01	5	-1.124e-2	1	408.202	2	1133.391	1
125	6	max	0	12	.132	3	.223	1	7.773e-3	3	NC	5	NC	3
126		min	-.153	4	-.466	2	.011	15	-1.231e-2	1	473.617	2	1168.083	1
127	7	max	0	12	.06	3	.177	1	8.534e-3	3	NC	5	NC	3
128		min	-.153	4	-.297	2	.014	12	-1.338e-2	1	688.075	2	1477.932	1
129	8	max	0	12	0	5	.104	1	9.294e-3	3	NC	3	NC	3
130		min	-.153	4	-.087	2	.007	10	-1.444e-2	1	1563.606	2	2516.284	1
131	9	max	0	12	.133	1	.044	4	1.005e-2	3	NC	2	NC	2
132		min	-.153	4	-.103	3	-.002	10	-1.551e-2	1	4854.603	3	5914.511	4
133	10	max	0	1	.219	1	.013	3	1.082e-2	3	NC	5	NC	1
134		min	-.153	4	-.137	3	-.009	2	-1.658e-2	1	2006.311	1	NC	1
135	11	max	0	1	.133	1	.032	1	1.005e-2	3	NC	2	NC	2
136		min	-.153	4	-.103	3	-.03	5	-1.551e-2	1	4854.603	3	8459.055	1
137	12	max	0	1	0	15	.104	1	9.294e-3	3	NC	3	NC	3
138		min	-.153	4	-.087	2	-.031	5	-1.444e-2	1	1563.606	2	2516.284	1
139	13	max	0	1	.06	3	.177	1	8.534e-3	3	NC	5	NC	3
140		min	-.153	4	-.297	2	-.014	5	-1.338e-2	1	688.075	2	1477.932	1
141	14	max	0	1	.132	3	.223	1	7.773e-3	3	NC	5	NC	3
142		min	-.153	4	-.466	2	.008	15	-1.231e-2	1	473.617	2	1168.083	1
143	15	max	0	1	.172	3	.23	1	7.013e-3	3	NC	5	NC	3
144		min	-.152	4	-.554	2	.014	12	-1.124e-2	1	408.202	2	1133.391	1
145	16	max	0	1	.171	3	.195	1	6.252e-3	3	NC	5	NC	3
146		min	-.152	4	-.536	2	.012	12	-1.018e-2	1	420.028	2	1337.124	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	.001	1	.128	3	.129	1	5.492e-3	3	NC	5	NC	3
148			min	-.152	4	-.41	2	.009	12	-9.109e-3	1	527.635	1	2034.044	1
149		18	max	.001	1	.05	3	.057	4	4.731e-3	3	NC	5	NC	2
150			min	-.152	4	-.193	2	.004	10	-8.042e-3	1	948.047	1	4524.833	4
151		19	max	.001	1	.091	1	.004	3	3.97e-3	3	NC	1	NC	1
152			min	-.152	4	-.05	3	-.002	2	-6.975e-3	1	NC	1	NC	1
153	M2	1	max	.006	1	.004	2	.01	1	1.529e-3	5	NC	1	NC	2
154			min	-.006	3	-.008	3	-.657	4	-2.726e-4	1	NC	1	95.536	4
155		2	max	.006	1	.003	2	.009	1	1.639e-3	5	NC	1	NC	2
156			min	-.005	3	-.008	3	-.603	4	-2.557e-4	1	NC	1	104.013	4
157		3	max	.005	1	.003	2	.008	1	1.748e-3	5	NC	1	NC	2
158			min	-.005	3	-.008	3	-.55	4	-2.388e-4	1	NC	1	114.077	4
159		4	max	.005	1	.002	2	.007	1	1.857e-3	5	NC	1	NC	2
160			min	-.005	3	-.007	3	-.497	4	-2.218e-4	1	NC	1	126.14	4
161		5	max	.005	1	.001	2	.007	1	1.966e-3	5	NC	1	NC	2
162			min	-.004	3	-.007	3	-.446	4	-2.049e-4	1	NC	1	140.762	4
163		6	max	.004	1	0	2	.006	1	2.075e-3	5	NC	1	NC	1
164			min	-.004	3	-.007	3	-.395	4	-1.88e-4	1	NC	1	158.718	4
165		7	max	.004	1	0	2	.005	1	2.185e-3	5	NC	1	NC	1
166			min	-.004	3	-.007	3	-.346	4	-1.711e-4	1	NC	1	181.107	4
167		8	max	.004	1	0	15	.004	1	2.294e-3	5	NC	1	NC	1
168			min	-.003	3	-.006	3	-.299	4	-1.541e-4	1	NC	1	209.527	4
169		9	max	.003	1	0	15	.004	1	2.403e-3	4	NC	1	NC	1
170			min	-.003	3	-.006	3	-.255	4	-1.372e-4	1	NC	1	246.381	4
171		10	max	.003	1	0	15	.003	1	2.519e-3	4	NC	1	NC	1
172			min	-.003	3	-.006	3	-.212	4	-1.203e-4	1	NC	1	295.416	4
173		11	max	.003	1	0	15	.002	1	2.634e-3	4	NC	1	NC	1
174			min	-.002	3	-.005	3	-.173	4	-1.034e-4	1	NC	1	362.746	4
175		12	max	.002	1	0	15	.002	1	2.749e-3	4	NC	1	NC	1
176			min	-.002	3	-.005	3	-.137	4	-8.644e-5	1	NC	1	458.91	4
177		13	max	.002	1	0	15	.001	1	2.865e-3	4	NC	1	NC	1
178			min	-.002	3	-.004	3	-.104	4	-6.951e-5	1	NC	1	603.393	4
179		14	max	.002	1	0	15	.001	1	2.98e-3	4	NC	1	NC	1
180			min	-.002	3	-.004	3	-.075	4	-5.259e-5	1	NC	1	835.625	4
181		15	max	.001	1	0	15	0	1	3.096e-3	4	NC	1	NC	1
182			min	-.001	3	-.003	3	-.05	4	-3.566e-5	1	NC	1	1245.852	4
183		16	max	0	1	0	15	0	1	3.211e-3	4	NC	1	NC	1
184			min	0	3	-.002	6	-.03	4	-1.873e-5	1	NC	1	2081.017	4
185		17	max	0	1	0	15	0	1	3.327e-3	4	NC	1	NC	1
186			min	0	3	-.002	6	-.015	4	-1.808e-6	1	NC	1	4241.977	4
187		18	max	0	1	0	15	0	1	3.442e-3	4	NC	1	NC	1
188			min	0	3	0	6	-.005	4	5.542e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	3.557e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.379e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-4.599e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-8.697e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.017	4	1.771e-5	1	NC	1	NC	1
194			min	0	2	-.002	6	0	12	-1.49e-4	5	NC	1	NC	1
195		3	max	0	3	0	15	.032	4	5.762e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	2.105e-6	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.047	4	1.299e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	3.388e-6	12	NC	1	8141.528	5
199		5	max	.001	3	-.002	15	.06	4	2.022e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	4.67e-6	12	NC	1	7172.64	5
201		6	max	.001	3	-.002	15	.073	4	2.745e-3	4	NC	1	NC	1
202			min	0	2	-.009	6	0	12	5.953e-6	12	NC	1	6866.818	5
203		7	max	.002	3	-.002	15	.085	4	3.468e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.001	2	-.01	6	0	12	7.235e-6	12	8738.544	6	7007.801	5
205		8	max	.002	3	-.003	15	.095	4	4.191e-3	4	NC	1	NC	1
206			min	-.001	2	-.012	6	0	12	8.518e-6	12	7836.824	6	7576.63	5
207		9	max	.002	3	-.003	15	.106	4	4.914e-3	4	NC	2	NC	1
208			min	-.001	2	-.013	6	0	12	9.8e-6	12	7302.675	6	8699.05	5
209		10	max	.002	3	-.003	15	.116	4	5.637e-3	4	NC	3	NC	1
210			min	-.002	2	-.013	6	0	12	1.108e-5	12	7042.789	6	NC	1
211		11	max	.003	3	-.003	15	.126	4	6.36e-3	4	NC	3	NC	1
212			min	-.002	2	-.013	6	0	12	1.237e-5	12	7018.793	6	NC	1
213		12	max	.003	3	-.003	15	.135	4	7.083e-3	4	NC	3	NC	1
214			min	-.002	2	-.013	6	0	12	1.365e-5	12	7232.524	6	NC	1
215		13	max	.003	3	-.003	15	.145	4	7.806e-3	4	NC	1	NC	1
216			min	-.002	2	-.012	6	0	12	1.493e-5	12	7728.603	6	NC	1
217		14	max	.003	3	-.002	15	.155	4	8.529e-3	4	NC	1	NC	1
218			min	-.002	2	-.011	6	0	12	1.621e-5	12	8617.674	6	NC	1
219		15	max	.004	3	-.002	15	.165	4	9.252e-3	4	NC	1	NC	1
220			min	-.003	2	-.009	6	0	12	1.75e-5	12	NC	1	NC	1
221		16	max	.004	3	-.001	15	.176	4	9.975e-3	4	NC	1	NC	1
222			min	-.003	2	-.008	1	0	12	1.878e-5	12	NC	1	NC	1
223		17	max	.004	3	0	15	.188	4	1.07e-2	4	NC	1	NC	1
224			min	-.003	2	-.006	1	0	12	2.006e-5	12	NC	1	NC	1
225		18	max	.004	3	0	15	.201	4	1.142e-2	4	NC	1	NC	1
226			min	-.003	2	-.005	1	0	12	2.134e-5	12	NC	1	NC	1
227		19	max	.005	3	0	5	.216	4	1.214e-2	4	NC	1	NC	2
228			min	-.003	2	-.003	1	0	12	2.263e-5	12	NC	1	9752.687	1
229	M4	1	max	.003	1	.003	2	0	12	6.629e-5	1	NC	1	NC	3
230			min	0	3	-.005	3	-.216	4	-4.343e-4	5	NC	1	115.056	4
231		2	max	.003	1	.003	2	0	12	6.629e-5	1	NC	1	NC	3
232			min	0	3	-.004	3	-.198	4	-4.343e-4	5	NC	1	125.134	4
233		3	max	.002	1	.003	2	0	12	6.629e-5	1	NC	1	NC	3
234			min	0	3	-.004	3	-.181	4	-4.343e-4	5	NC	1	137.125	4
235		4	max	.002	1	.002	2	0	12	6.629e-5	1	NC	1	NC	3
236			min	0	3	-.004	3	-.164	4	-4.343e-4	5	NC	1	151.528	4
237		5	max	.002	1	.002	2	0	12	6.629e-5	1	NC	1	NC	3
238			min	0	3	-.004	3	-.147	4	-4.343e-4	5	NC	1	169.019	4
239		6	max	.002	1	.002	2	0	12	6.629e-5	1	NC	1	NC	2
240			min	0	3	-.003	3	-.13	4	-4.343e-4	5	NC	1	190.535	4
241		7	max	.002	1	.002	2	0	12	6.629e-5	1	NC	1	NC	2
242			min	0	3	-.003	3	-.114	4	-4.343e-4	5	NC	1	217.408	4
243		8	max	.002	1	.002	2	0	12	6.629e-5	1	NC	1	NC	2
244			min	0	3	-.003	3	-.099	4	-4.343e-4	5	NC	1	251.577	4
245		9	max	.001	1	.002	2	0	12	6.629e-5	1	NC	1	NC	2
246			min	0	3	-.003	3	-.084	4	-4.343e-4	5	NC	1	295.965	4
247		10	max	.001	1	.001	2	0	12	6.629e-5	1	NC	1	NC	2
248			min	0	3	-.002	3	-.07	4	-4.343e-4	5	NC	1	355.139	4
249		11	max	.001	1	.001	2	0	12	6.629e-5	1	NC	1	NC	1
250			min	0	3	-.002	3	-.057	4	-4.343e-4	5	NC	1	436.571	4
251		12	max	.001	1	.001	2	0	12	6.629e-5	1	NC	1	NC	1
252			min	0	3	-.002	3	-.045	4	-4.343e-4	5	NC	1	553.186	4
253		13	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.034	4	-4.343e-4	5	NC	1	728.972	4
255		14	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
256			min	0	3	-.001	3	-.024	4	-4.343e-4	5	NC	1	1012.73	4
257		15	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
258			min	0	3	-.001	3	-.016	4	-4.343e-4	5	NC	1	1516.951	4
259		16	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
260			min	0	3	0	3	-.01	4	-4.343e-4	5	NC	1	2552.578	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
262		min	0	3	0	3	-.005	4	-4.343e-4	5	NC	1	5272.415	4
263	18	max	0	1	0	2	0	12	6.629e-5	1	NC	1	NC	1
264		min	0	3	0	3	-.001	4	-4.343e-4	5	NC	1	NC	1
265	19	max	0	1	0	1	0	1	6.629e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	-4.343e-4	5	NC	1	NC	1
267	M6	1	max	.019	1	.018	2	0	1.624e-3	4	NC	3	NC	1
268		min	-.018	3	-.025	3	-.663	4	0	1	3497.834	2	94.638	4
269	2	max	.018	1	.016	2	0	1	1.731e-3	4	NC	3	NC	1
270		min	-.017	3	-.024	3	-.609	4	0	1	3853.061	2	103.037	4
271	3	max	.017	1	.015	2	0	1	1.838e-3	4	NC	3	NC	1
272		min	-.016	3	-.023	3	-.555	4	0	1	4284.7	2	113.008	4
273	4	max	.016	1	.013	2	0	1	1.945e-3	4	NC	3	NC	1
274		min	-.015	3	-.021	3	-.502	4	0	1	4815.495	2	124.961	4
275	5	max	.015	1	.011	2	0	1	2.052e-3	4	NC	1	NC	1
276		min	-.014	3	-.02	3	-.45	4	0	1	5477.633	2	139.45	4
277	6	max	.014	1	.01	2	0	1	2.16e-3	4	NC	1	NC	1
278		min	-.013	3	-.018	3	-.399	4	0	1	6317.898	2	157.243	4
279	7	max	.013	1	.008	2	0	1	2.267e-3	4	NC	1	NC	1
280		min	-.012	3	-.017	3	-.35	4	0	1	7406.461	2	179.429	4
281	8	max	.011	1	.007	2	0	1	2.374e-3	4	NC	1	NC	1
282		min	-.011	3	-.016	3	-.302	4	0	1	8852.603	2	207.593	4
283	9	max	.01	1	.006	2	0	1	2.481e-3	4	NC	1	NC	1
284		min	-.01	3	-.014	3	-.257	4	0	1	NC	1	244.118	4
285	10	max	.009	1	.005	2	0	1	2.588e-3	4	NC	1	NC	1
286		min	-.009	3	-.013	3	-.214	4	0	1	NC	1	292.717	4
287	11	max	.008	1	.004	2	0	1	2.696e-3	4	NC	1	NC	1
288		min	-.008	3	-.012	3	-.174	4	0	1	NC	1	359.454	4
289	12	max	.007	1	.003	2	0	1	2.803e-3	4	NC	1	NC	1
290		min	-.007	3	-.01	3	-.138	4	0	1	NC	1	454.78	4
291	13	max	.006	1	.002	2	0	1	2.91e-3	4	NC	1	NC	1
292		min	-.006	3	-.009	3	-.105	4	0	1	NC	1	598.023	4
293	14	max	.005	1	.001	2	0	1	3.017e-3	4	NC	1	NC	1
294		min	-.005	3	-.007	3	-.076	4	0	1	NC	1	828.297	4
295	15	max	.004	1	0	2	0	1	3.124e-3	4	NC	1	NC	1
296		min	-.004	3	-.006	3	-.051	4	0	1	NC	1	1235.159	4
297	16	max	.003	1	0	2	0	1	3.232e-3	4	NC	1	NC	1
298		min	-.003	3	-.004	3	-.03	4	0	1	NC	1	2063.749	4
299	17	max	.002	1	0	2	0	1	3.339e-3	4	NC	1	NC	1
300		min	-.002	3	-.003	3	-.015	4	0	1	NC	1	4208.898	4
301	18	max	.001	1	0	2	0	1	3.446e-3	4	NC	1	NC	1
302		min	-.001	3	-.001	3	-.005	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	3.553e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-8.675e-4	4	NC	1	NC	1
307	2	max	0	3	0	15	.017	4	0	1	NC	1	NC	1
308		min	0	2	-.002	3	0	1	-1.63e-4	4	NC	1	NC	1
309	3	max	.002	3	0	15	.032	4	5.415e-4	4	NC	1	NC	1
310		min	-.002	2	-.004	3	0	1	0	1	NC	1	9844.906	4
311	4	max	.002	3	-.001	15	.047	4	1.246e-3	4	NC	1	NC	1
312		min	-.002	2	-.006	3	0	1	0	1	NC	1	7544.2	4
313	5	max	.003	3	-.002	15	.06	4	1.951e-3	4	NC	1	NC	1
314		min	-.003	2	-.008	3	0	1	0	1	NC	1	6595.054	4
315	6	max	.004	3	-.002	15	.073	4	2.655e-3	4	NC	1	NC	1
316		min	-.004	2	-.01	3	0	1	0	1	NC	1	6252.916	4
317	7	max	.005	3	-.003	15	.084	4	3.36e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.005	2	-.011	3	0	1	0	1	8822.972	4	6302.232	4
319	8	max	.006	3	-.003	15	.095	4	4.064e-3	4	NC	1	NC	1
320		min	-.005	2	-.012	4	0	1	0	1	7906.973	4	6701.593	4
321	9	max	.006	3	-.003	15	.105	4	4.769e-3	4	NC	1	NC	1
322		min	-.006	2	-.013	4	0	1	0	1	7363.742	4	7518.009	4
323	10	max	.007	3	-.003	15	.115	4	5.473e-3	4	NC	1	NC	1
324		min	-.007	2	-.014	4	0	1	0	1	7098.216	4	8958.288	4
325	11	max	.008	3	-.003	15	.124	4	6.178e-3	4	NC	1	NC	1
326		min	-.008	2	-.014	4	0	1	0	1	7071.129	4	NC	1
327	12	max	.009	3	-.003	15	.133	4	6.882e-3	4	NC	1	NC	1
328		min	-.008	2	-.013	4	0	1	0	1	7283.936	4	NC	1
329	13	max	.01	3	-.003	15	.142	4	7.587e-3	4	NC	1	NC	1
330		min	-.009	2	-.013	4	0	1	0	1	7781.287	4	NC	1
331	14	max	.01	3	-.003	15	.152	4	8.291e-3	4	NC	1	NC	1
332		min	-.01	2	-.012	4	0	1	0	1	8674.33	4	NC	1
333	15	max	.011	3	-.002	15	.162	4	8.996e-3	4	NC	1	NC	1
334		min	-.011	2	-.01	4	0	1	0	1	NC	1	NC	1
335	16	max	.012	3	-.002	15	.172	4	9.7e-3	4	NC	1	NC	1
336		min	-.011	2	-.009	1	0	1	0	1	NC	1	NC	1
337	17	max	.013	3	-.001	15	.183	4	1.04e-2	4	NC	1	NC	1
338		min	-.012	2	-.008	1	0	1	0	1	NC	1	NC	1
339	18	max	.013	3	0	15	.196	4	1.111e-2	4	NC	1	NC	1
340		min	-.013	2	-.007	1	0	1	0	1	NC	1	NC	1
341	19	max	.014	3	0	15	.209	4	1.181e-2	4	NC	1	NC	1
342		min	-.014	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.013	2	0	0	1	NC	1	NC	1
344		min	0	3	-.015	3	-.209	4	-5.142e-4	4	NC	1	118.616	4
345	2	max	.007	1	.012	2	0	1	0	1	NC	1	NC	1
346		min	0	3	-.014	3	-.192	4	-5.142e-4	4	NC	1	129.011	4
347	3	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
348		min	0	3	-.013	3	-.175	4	-5.142e-4	4	NC	1	141.38	4
349	4	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
350		min	0	3	-.012	3	-.159	4	-5.142e-4	4	NC	1	156.237	4
351	5	max	.006	1	.01	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.011	3	-.142	4	-5.142e-4	4	NC	1	174.278	4
353	6	max	.005	1	.009	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.011	3	-.126	4	-5.142e-4	4	NC	1	196.471	4
355	7	max	.005	1	.008	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.01	3	-.111	4	-5.142e-4	4	NC	1	224.188	4
357	8	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.009	3	-.096	4	-5.142e-4	4	NC	1	259.431	4
359	9	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.008	3	-.081	4	-5.142e-4	4	NC	1	305.214	4
361	10	max	.004	1	.006	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.007	3	-.068	4	-5.142e-4	4	NC	1	366.249	4
363	11	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.006	3	-.055	4	-5.142e-4	4	NC	1	450.241	4
365	12	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.006	3	-.043	4	-5.142e-4	4	NC	1	570.523	4
367	13	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.005	3	-.033	4	-5.142e-4	4	NC	1	751.839	4
369	14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.004	3	-.024	4	-5.142e-4	4	NC	1	1044.525	4
371	15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.003	3	-.016	4	-5.142e-4	4	NC	1	1564.617	4
373	16	max	.001	1	.002	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.009	4	-5.142e-4	4	NC	1	2632.861	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.005	4	-5.142e-4	4	NC	1	5438.419	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-5.142e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-5.142e-4	4	NC	1	NC	1
381	M10	1	max	.006	1	.004	2	0	12	1.635e-3	4	NC	1	NC	2
382			min	-.006	3	-.008	3	-.662	4	1.346e-5	12	NC	1	94.736	4
383		2	max	.006	1	.003	2	0	12	1.741e-3	4	NC	1	NC	2
384			min	-.005	3	-.008	3	-.608	4	1.264e-5	12	NC	1	103.144	4
385		3	max	.005	1	.003	2	0	12	1.846e-3	4	NC	1	NC	2
386			min	-.005	3	-.008	3	-.554	4	1.181e-5	12	NC	1	113.126	4
387		4	max	.005	1	.002	2	0	12	1.952e-3	4	NC	1	NC	2
388			min	-.005	3	-.007	3	-.501	4	1.099e-5	12	NC	1	125.091	4
389		5	max	.005	1	.001	2	0	12	2.058e-3	4	NC	1	NC	2
390			min	-.004	3	-.007	3	-.449	4	1.016e-5	12	NC	1	139.596	4
391		6	max	.004	1	0	2	0	12	2.164e-3	4	NC	1	NC	1
392			min	-.004	3	-.007	3	-.398	4	9.339e-6	12	NC	1	157.408	4
393		7	max	.004	1	0	2	0	12	2.27e-3	4	NC	1	NC	1
394			min	-.004	3	-.007	3	-.349	4	8.514e-6	12	NC	1	179.619	4
395		8	max	.004	1	0	2	0	12	2.375e-3	4	NC	1	NC	1
396			min	-.003	3	-.006	3	-.302	4	7.69e-6	12	NC	1	207.815	4
397		9	max	.003	1	0	2	0	12	2.481e-3	4	NC	1	NC	1
398			min	-.003	3	-.006	3	-.257	4	6.865e-6	12	NC	1	244.38	4
399		10	max	.003	1	-.001	2	0	12	2.587e-3	4	NC	1	NC	1
400			min	-.003	3	-.006	3	-.214	4	6.041e-6	12	NC	1	293.035	4
401		11	max	.003	1	-.001	15	0	12	2.693e-3	4	NC	1	NC	1
402			min	-.002	3	-.005	3	-.174	4	5.217e-6	12	NC	1	359.849	4
403		12	max	.002	1	-.001	15	0	12	2.798e-3	4	NC	1	NC	1
404			min	-.002	3	-.005	3	-.138	4	4.392e-6	12	NC	1	455.289	4
405		13	max	.002	1	-.001	15	0	12	2.904e-3	4	NC	1	NC	1
406			min	-.002	3	-.004	4	-.105	4	3.568e-6	12	NC	1	598.706	4
407		14	max	.002	1	-.001	15	0	12	3.01e-3	4	NC	1	NC	1
408			min	-.002	3	-.004	4	-.076	4	2.743e-6	12	NC	1	829.272	4
409		15	max	.001	1	0	15	0	12	3.116e-3	4	NC	1	NC	1
410			min	-.001	3	-.003	4	-.051	4	1.919e-6	12	NC	1	1236.675	4
411		16	max	0	1	0	15	0	12	3.222e-3	4	NC	1	NC	1
412			min	0	3	-.003	4	-.03	4	1.095e-6	12	NC	1	2066.443	4
413		17	max	0	1	0	15	0	12	3.327e-3	4	NC	1	NC	1
414			min	0	3	-.002	4	-.015	4	1.272e-7	10	NC	1	4214.984	4
415		18	max	0	1	0	15	0	12	3.433e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.005	4	-1.512e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.539e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-3.204e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.054e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-8.637e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.017	4	-8.226e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.568e-4	4	NC	1	NC	1
423		3	max	0	3	0	15	.032	4	5.502e-4	5	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-4.596e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.047	4	1.257e-3	4	NC	1	NC	1
426			min	0	2	-.006	4	0	1	-7.421e-5	1	NC	1	7799.106	4
427		5	max	.001	3	-.002	15	.06	4	1.964e-3	4	NC	1	NC	1
428			min	0	2	-.008	4	0	1	-1.025e-4	1	NC	1	6849.392	4
429		6	max	.001	3	-.002	15	.072	4	2.671e-3	4	NC	1	NC	1
430			min	0	2	-.01	4	-.001	1	-1.307e-4	1	9854.649	4	6531.142	4
431		7	max	.002	3	-.003	15	.084	4	3.378e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.001	2	-.011	4	-.001	1	-1.59e-4	1	8466.17	4	6630.418	4
433		8	max	.002	3	-.003	15	.094	4	4.085e-3	4	NC	1	NC	1
434			min	-.001	2	-.012	4	-.002	1	-1.872e-4	1	7609.843	4	7117.934	4
435		9	max	.002	3	-.003	15	.105	4	4.791e-3	4	NC	2	NC	1
436			min	-.001	2	-.013	4	-.002	1	-2.155e-4	1	7104.588	4	8090.333	4
437		10	max	.002	3	-.003	15	.114	4	5.498e-3	4	NC	3	NC	1
438			min	-.002	2	-.014	4	-.003	1	-2.437e-4	1	6862.624	4	9827.487	4
439		11	max	.003	3	-.003	15	.124	4	6.205e-3	4	NC	3	NC	1
440			min	-.002	2	-.014	4	-.003	1	-2.719e-4	1	6848.381	4	NC	1
441		12	max	.003	3	-.003	15	.133	4	6.912e-3	4	NC	3	NC	1
442			min	-.002	2	-.014	4	-.004	1	-3.002e-4	1	7064.871	4	NC	1
443		13	max	.003	3	-.003	15	.142	4	7.619e-3	4	NC	1	NC	1
444			min	-.002	2	-.013	4	-.004	1	-3.284e-4	1	7556.592	4	NC	1
445		14	max	.003	3	-.003	15	.152	4	8.326e-3	4	NC	1	NC	1
446			min	-.002	2	-.012	4	-.005	1	-3.567e-4	1	8432.502	4	NC	1
447		15	max	.004	3	-.003	15	.162	4	9.033e-3	4	NC	1	NC	1
448			min	-.003	2	-.01	4	-.006	1	-3.849e-4	1	9933.716	4	NC	1
449		16	max	.004	3	-.002	15	.173	4	9.74e-3	4	NC	1	NC	1
450			min	-.003	2	-.008	4	-.006	1	-4.132e-4	1	NC	1	NC	1
451		17	max	.004	3	-.002	15	.184	4	1.045e-2	4	NC	1	NC	1
452			min	-.003	2	-.006	1	-.007	1	-4.414e-4	1	NC	1	NC	1
453		18	max	.004	3	-.001	15	.196	4	1.115e-2	4	NC	1	NC	1
454			min	-.003	2	-.005	1	-.008	1	-4.697e-4	1	NC	1	NC	1
455		19	max	.005	3	0	10	.21	4	1.186e-2	4	NC	1	NC	2
456			min	-.003	2	-.003	1	-.009	1	-4.979e-4	1	NC	1	9752.687	1
457	M12	1	max	.003	1	.003	2	.009	1	-3.18e-6	12	NC	1	NC	3
458			min	0	3	-.005	3	-.21	4	-4.644e-4	4	NC	1	117.945	4
459		2	max	.003	1	.003	2	.009	1	-3.18e-6	12	NC	1	NC	3
460			min	0	3	-.004	3	-.193	4	-4.644e-4	4	NC	1	128.276	4
461		3	max	.002	1	.003	2	.008	1	-3.18e-6	12	NC	1	NC	3
462			min	0	3	-.004	3	-.176	4	-4.644e-4	4	NC	1	140.57	4
463		4	max	.002	1	.002	2	.007	1	-3.18e-6	12	NC	1	NC	3
464			min	0	3	-.004	3	-.16	4	-4.644e-4	4	NC	1	155.336	4
465		5	max	.002	1	.002	2	.006	1	-3.18e-6	12	NC	1	NC	3
466			min	0	3	-.004	3	-.143	4	-4.644e-4	4	NC	1	173.267	4
467		6	max	.002	1	.002	2	.006	1	-3.18e-6	12	NC	1	NC	2
468			min	0	3	-.003	3	-.127	4	-4.644e-4	4	NC	1	195.325	4
469		7	max	.002	1	.002	2	.005	1	-3.18e-6	12	NC	1	NC	2
470			min	0	3	-.003	3	-.111	4	-4.644e-4	4	NC	1	222.874	4
471		8	max	.002	1	.002	2	.004	1	-3.18e-6	12	NC	1	NC	2
472			min	0	3	-.003	3	-.096	4	-4.644e-4	4	NC	1	257.903	4
473		9	max	.001	1	.002	2	.004	1	-3.18e-6	12	NC	1	NC	2
474			min	0	3	-.003	3	-.082	4	-4.644e-4	4	NC	1	303.408	4
475		10	max	.001	1	.001	2	.003	1	-3.18e-6	12	NC	1	NC	2
476			min	0	3	-.002	3	-.068	4	-4.644e-4	4	NC	1	364.071	4
477		11	max	.001	1	.001	2	.002	1	-3.18e-6	12	NC	1	NC	1
478			min	0	3	-.002	3	-.055	4	-4.644e-4	4	NC	1	447.553	4
479		12	max	.001	1	.001	2	.002	1	-3.18e-6	12	NC	1	NC	1
480			min	0	3	-.002	3	-.044	4	-4.644e-4	4	NC	1	567.103	4
481		13	max	0	1	0	2	.001	1	-3.18e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.033	4	-4.644e-4	4	NC	1	747.313	4
483		14	max	0	1	0	2	.001	1	-3.18e-6	12	NC	1	NC	1
484			min	0	3	-.001	3	-.024	4	-4.644e-4	4	NC	1	1038.213	4
485		15	max	0	1	0	2	0	1	-3.18e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.016	4	-4.644e-4	4	NC	1	1555.124	4
487		16	max	0	1	0	2	0	1	-3.18e-6	12	NC	1	NC	1
488			min	0	3	0	3	-.009	4	-4.644e-4	4	NC	1	2616.82	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-3.18e-6	12	NC	1	NC	1
490			min	0	3	0	3	-.005	4	-4.644e-4	4	NC	1	5405.127	4
491		18	max	0	1	0	2	0	1	-3.18e-6	12	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-4.644e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-3.18e-6	12	NC	1	NC	1
494			min	0	1	0	1	0	1	-4.644e-4	4	NC	1	NC	1
495	M1	1	max	.006	3	.092	1	.701	4	1.699e-2	1	NC	1	NC	1
496			min	-.002	2	-.008	3	0	12	-2.192e-2	3	NC	1	NC	1
497		2	max	.006	3	.045	1	.679	4	9.33e-3	4	NC	3	NC	1
498			min	-.002	2	-.002	3	-.007	1	-1.085e-2	3	2407.919	1	NC	1
499		3	max	.006	3	.008	3	.656	4	1.495e-2	4	NC	5	NC	1
500			min	-.002	2	-.007	2	-.01	1	-2.036e-4	1	1151.267	1	6396.007	5
501		4	max	.006	3	.029	3	.633	4	1.312e-2	4	NC	5	NC	1
502			min	-.002	2	-.066	1	-.009	1	-3.771e-3	3	718.651	1	4545.231	5
503		5	max	.006	3	.056	3	.61	4	1.129e-2	4	NC	15	NC	1
504			min	-.002	2	-.13	1	-.006	1	-7.435e-3	3	513.809	1	3614.799	5
505		6	max	.005	3	.087	3	.586	4	1.376e-2	1	NC	15	NC	1
506			min	-.002	2	-.192	1	-.003	1	-1.11e-2	3	401.773	1	3057.863	5
507		7	max	.005	3	.116	3	.561	4	1.841e-2	1	9896.054	15	NC	1
508			min	-.002	2	-.247	1	0	12	-1.476e-2	3	336.033	1	2673.762	4
509		8	max	.005	3	.141	3	.535	4	2.306e-2	1	8786.607	15	NC	1
510			min	-.002	2	-.292	1	0	12	-1.843e-2	3	297.316	1	2398.944	4
511		9	max	.005	3	.157	3	.508	4	2.543e-2	1	8208.642	15	NC	1
512			min	-.002	2	-.319	1	0	1	-1.837e-2	3	277.224	1	2234.621	4
513		10	max	.005	3	.163	3	.479	4	2.628e-2	1	8032.721	15	NC	1
514			min	-.002	2	-.329	1	0	12	-1.584e-2	3	271.207	1	2190.497	4
515		11	max	.005	3	.159	3	.447	4	2.712e-2	1	8208.395	15	NC	1
516			min	-.002	2	-.319	1	0	12	-1.33e-2	3	277.567	1	2247.335	4
517		12	max	.005	3	.145	3	.413	4	2.563e-2	1	8786.044	15	NC	1
518			min	-.002	2	-.291	1	-.001	1	-1.091e-2	3	298.394	1	2421.262	4
519		13	max	.005	3	.124	3	.375	4	2.063e-2	1	9894.982	15	NC	1
520			min	-.002	2	-.245	1	0	1	-8.732e-3	3	338.715	1	2851.37	4
521		14	max	.005	3	.096	3	.335	4	1.563e-2	1	NC	15	NC	1
522			min	-.002	2	-.188	1	0	12	-6.553e-3	3	407.575	1	3737.785	4
523		15	max	.004	3	.065	3	.294	4	1.063e-2	1	NC	15	NC	1
524			min	-.002	2	-.126	1	0	12	-4.373e-3	3	525.84	1	5647.827	4
525		16	max	.004	3	.033	3	.254	4	1.e-2	4	NC	5	NC	1
526			min	-.002	2	-.062	1	0	12	-2.194e-3	3	744.149	1	NC	1
527		17	max	.004	3	.003	3	.216	4	1.113e-2	4	NC	5	NC	1
528			min	-.002	2	-.004	2	0	12	-1.408e-5	3	1209.295	1	NC	1
529		18	max	.004	3	.046	1	.183	4	1.002e-2	1	NC	4	NC	1
530			min	-.002	2	-.024	3	0	12	-3.562e-3	3	2555.858	1	NC	1
531		19	max	.004	3	.091	1	.152	4	1.974e-2	1	NC	1	NC	1
532			min	-.002	2	-.05	3	-.001	1	-7.243e-3	3	NC	1	NC	1
533	M5	1	max	.018	3	.224	1	.701	4	0	1	NC	1	NC	1
534			min	-.012	2	-.005	3	0	1	-4.133e-6	4	NC	1	NC	1
535		2	max	.018	3	.108	1	.683	4	7.676e-3	4	NC	5	NC	1
536			min	-.012	2	.002	3	0	1	0	1	983.466	1	8895.344	4
537		3	max	.018	3	.027	3	.662	4	1.512e-2	4	NC	15	NC	1
538			min	-.012	2	-.024	1	0	1	0	1	460.351	1	5160.489	4
539		4	max	.017	3	.084	3	.639	4	1.232e-2	4	9218.1	15	NC	1
540			min	-.011	2	-.184	1	0	1	0	1	279.819	1	3938.297	4
541		5	max	.017	3	.165	3	.613	4	9.517e-3	4	6455.16	15	NC	1
542			min	-.011	2	-.359	1	0	1	0	1	195.862	1	3341.27	4
543		6	max	.017	3	.257	3	.587	4	6.716e-3	4	4972.198	15	NC	1
544			min	-.011	2	-.533	1	0	1	0	1	150.778	1	2974.917	4
545		7	max	.016	3	.347	3	.561	4	3.916e-3	4	4115.309	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.011	2	-.692	1	0	1	0	1	124.717	1	2696.612	4
547		8	max	.016	3	.422	3	.535	4	1.115e-3	4	3616.861	15	NC	1
548			min	-.01	2	-.818	1	0	1	0	1	109.554	1	2439.937	4
549		9	max	.016	3	.471	3	.509	4	0	1	3361.164	15	NC	1
550			min	-.01	2	-.898	1	0	1	-2.774e-6	5	101.778	1	2230.87	4
551		10	max	.015	3	.489	3	.479	4	0	1	3284.119	15	NC	1
552			min	-.01	2	-.924	1	0	1	-2.677e-6	5	99.462	1	2204.839	4
553		11	max	.015	3	.476	3	.447	4	0	1	3361.245	15	NC	1
554			min	-.01	2	-.897	1	0	1	-2.58e-6	5	101.916	1	2272.363	4
555		12	max	.015	3	.435	3	.414	4	7.916e-4	4	3617.056	15	NC	1
556			min	-.01	2	-.815	1	0	1	0	1	110.009	1	2377.977	4
557		13	max	.014	3	.369	3	.376	4	2.781e-3	4	4115.717	15	NC	1
558			min	-.01	2	-.685	1	0	1	0	1	125.9	1	2807.379	4
559		14	max	.014	3	.286	3	.334	4	4.77e-3	4	4973.013	15	NC	1
560			min	-.01	2	-.522	1	0	1	0	1	153.437	1	3913.808	4
561		15	max	.014	3	.193	3	.291	4	6.759e-3	4	6456.792	15	NC	1
562			min	-.009	2	-.345	1	0	1	0	1	201.615	1	7068.532	4
563		16	max	.013	3	.098	3	.248	4	8.748e-3	4	9221.546	15	NC	1
564			min	-.009	2	-.169	1	0	1	0	1	292.663	1	NC	1
565		17	max	.013	3	.009	3	.21	4	1.074e-2	4	NC	15	NC	1
566			min	-.009	2	-.013	2	0	1	0	1	491.452	1	NC	1
567		18	max	.013	3	.113	1	.178	4	5.452e-3	4	NC	5	NC	1
568			min	-.009	2	-.068	3	0	1	0	1	1066.522	1	NC	1
569		19	max	.013	3	.219	1	.153	4	0	1	NC	1	NC	1
570			min	-.009	2	-.137	3	0	1	-2.294e-6	4	NC	1	NC	1
571	M9	1	max	.006	3	.092	1	.701	4	2.192e-2	3	NC	1	NC	1
572			min	-.002	2	-.008	3	-.001	1	-1.699e-2	1	NC	1	NC	1
573		2	max	.006	3	.045	1	.683	4	1.085e-2	3	NC	3	NC	1
574			min	-.002	2	-.002	3	0	12	-8.238e-3	1	2407.919	1	9101.727	4
575		3	max	.006	3	.008	3	.662	4	1.509e-2	4	NC	5	NC	1
576			min	-.002	2	-.007	2	0	12	-1.074e-5	10	1151.267	1	5234.828	4
577		4	max	.006	3	.029	3	.638	4	1.182e-2	5	NC	5	NC	1
578			min	-.002	2	-.066	1	0	12	-4.45e-3	1	718.651	1	3957.531	4
579		5	max	.006	3	.056	3	.613	4	8.873e-3	5	NC	15	NC	1
580			min	-.002	2	-.13	1	0	12	-9.103e-3	1	513.809	1	3329.53	4
581		6	max	.005	3	.087	3	.587	4	1.11e-2	3	NC	15	NC	1
582			min	-.002	2	-.192	1	0	12	-1.376e-2	1	401.773	1	2947.446	4
583		7	max	.005	3	.116	3	.561	4	1.476e-2	3	9875.526	15	NC	1
584			min	-.002	2	-.247	1	0	1	-1.841e-2	1	336.033	1	2667.719	4
585		8	max	.005	3	.141	3	.535	4	1.843e-2	3	8768.815	15	NC	1
586			min	-.002	2	-.292	1	-.001	1	-2.306e-2	1	297.316	1	2423.834	4
587		9	max	.005	3	.157	3	.508	4	1.837e-2	3	8192.245	15	NC	1
588			min	-.002	2	-.319	1	0	12	-2.543e-2	1	277.224	1	2227.766	4
589		10	max	.005	3	.163	3	.479	4	1.584e-2	3	8016.736	15	NC	1
590			min	-.002	2	-.329	1	0	1	-2.628e-2	1	271.207	1	2191.758	4
591		11	max	.005	3	.159	3	.447	4	1.33e-2	3	8192.016	15	NC	1
592			min	-.002	2	-.319	1	0	1	-2.712e-2	1	277.567	1	2256.359	4
593		12	max	.005	3	.145	3	.413	4	1.091e-2	3	8768.386	15	NC	1
594			min	-.002	2	-.291	1	0	12	-2.563e-2	1	298.394	1	2396.592	4
595		13	max	.005	3	.124	3	.375	4	8.732e-3	3	9874.855	15	NC	1
596			min	-.002	2	-.245	1	0	12	-2.063e-2	1	338.715	1	2853.742	4
597		14	max	.005	3	.096	3	.334	4	6.553e-3	3	NC	15	NC	1
598			min	-.002	2	-.188	1	-.002	1	-1.563e-2	1	407.575	1	3885.128	5
599		15	max	.004	3	.065	3	.291	4	6.341e-3	5	NC	15	NC	1
600			min	-.002	2	-.126	1	-.006	1	-1.063e-2	1	525.84	1	6380.043	5
601		16	max	.004	3	.033	3	.249	4	8.542e-3	5	NC	5	NC	1
602			min	-.002	2	-.062	1	-.009	1	-5.63e-3	1	744.149	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.004	3	.003	3	.211	4	1.079e-2	4	NC	5	NC	1
604		min	-.002	2	-.004	2	-.009	1	-6.284e-4	1	1209.295	1	NC	1
605	18	max	.004	3	.046	1	.179	4	5.068e-3	5	NC	4	NC	1
606		min	-.002	2	-.024	3	-.006	1	-1.002e-2	1	2555.858	1	NC	1
607	19	max	.004	3	.091	1	.153	4	7.243e-3	3	NC	1	NC	1
608		min	-.002	2	-.05	3	0	12	-1.974e-2	1	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

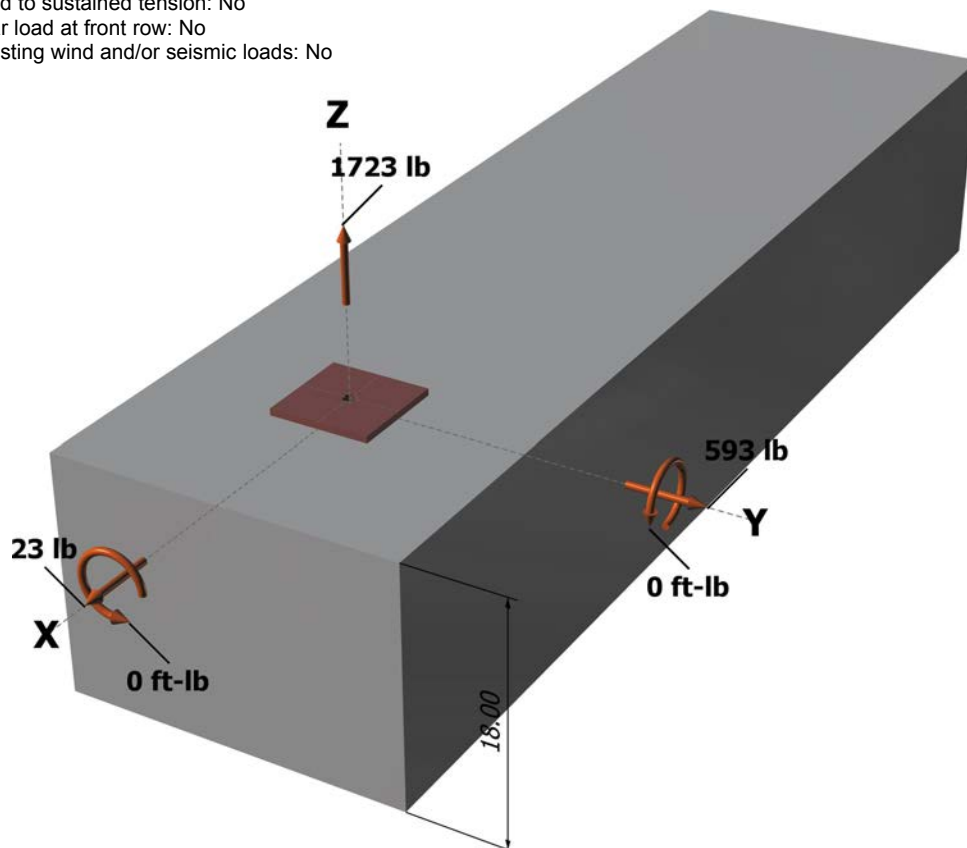
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

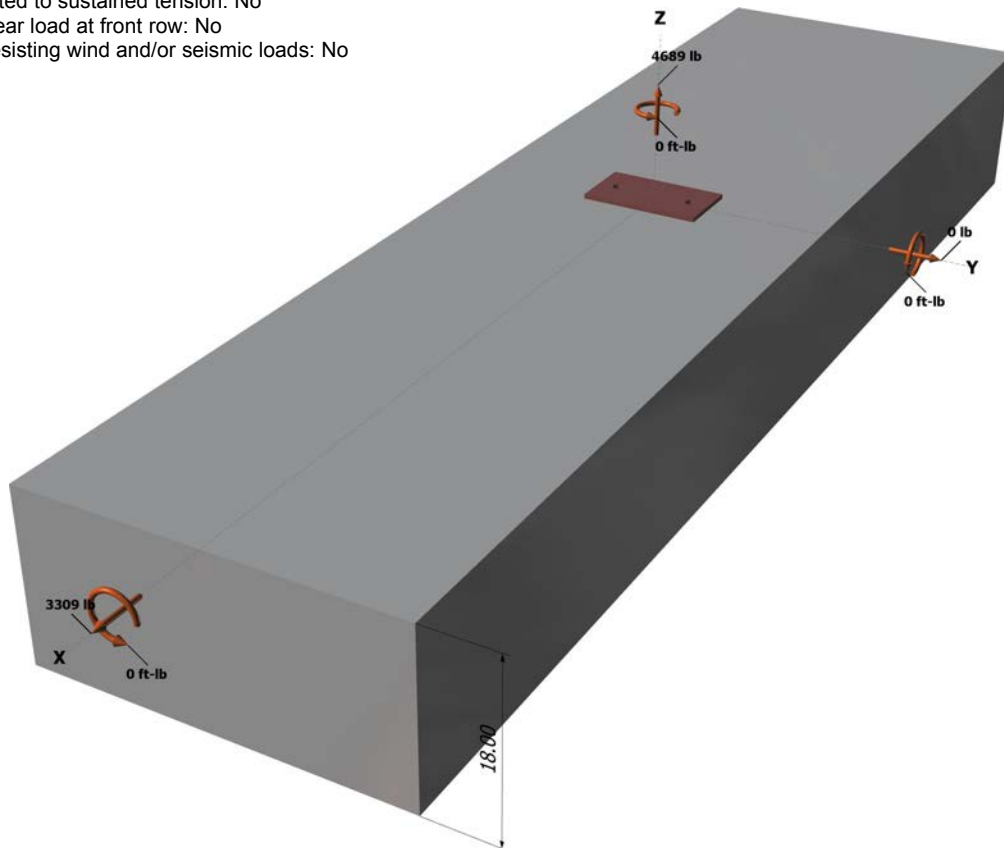
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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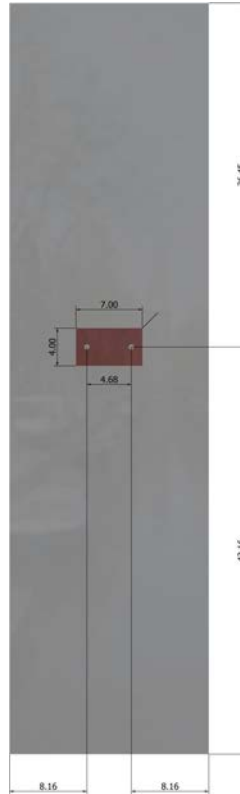




Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

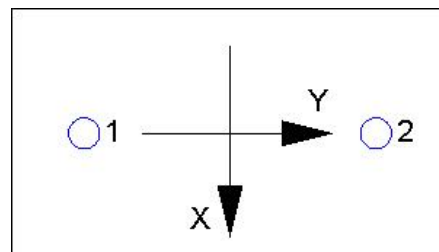
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.