



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	100 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II
Peak Velocity Pressure, q_z =	15.70 psf	Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	
S_{DS} =	0.00	C_s = 0	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

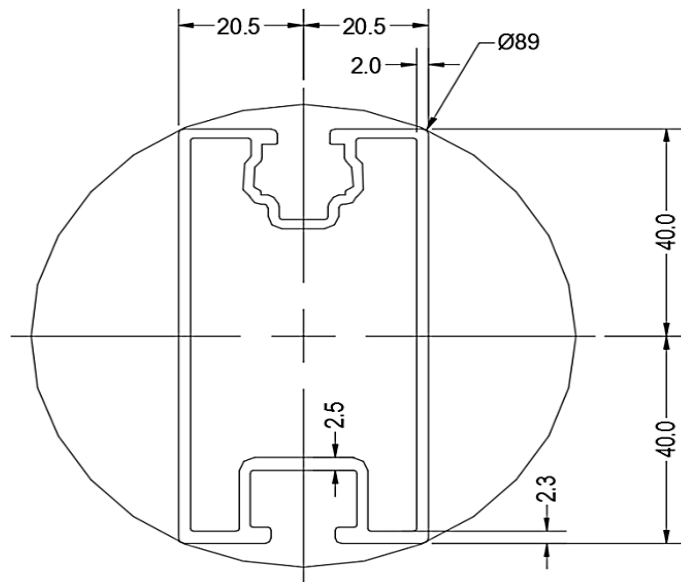
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

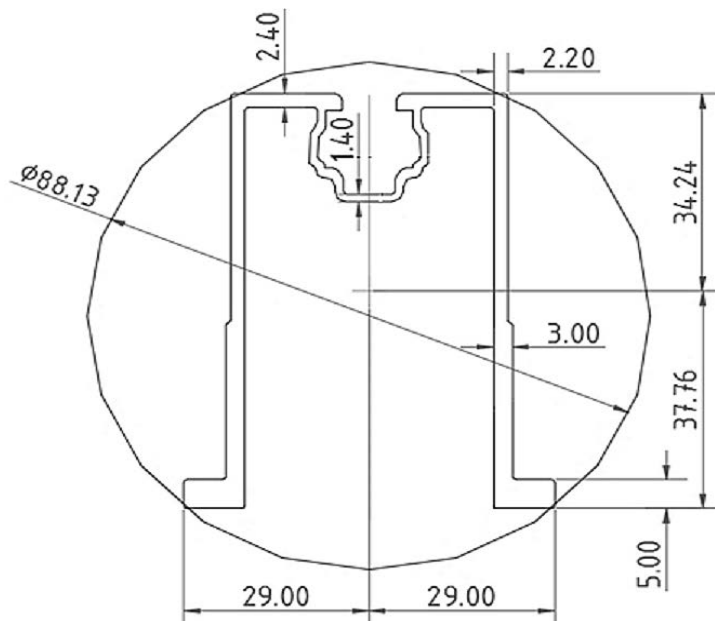
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	66 in
ΦF_{ty} STRONG-AXIS =	29.56 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.466 k-ft
M_z =	0.128 k-ft
$M_{y \text{ allowable}}$ =	1.837 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	41%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.38 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.574 k-ft
M_z =	0.000 k-ft
P_n =	0.280 k
$M_{y \text{ allowable}}$ =	1.442 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	42%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.816 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	7%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

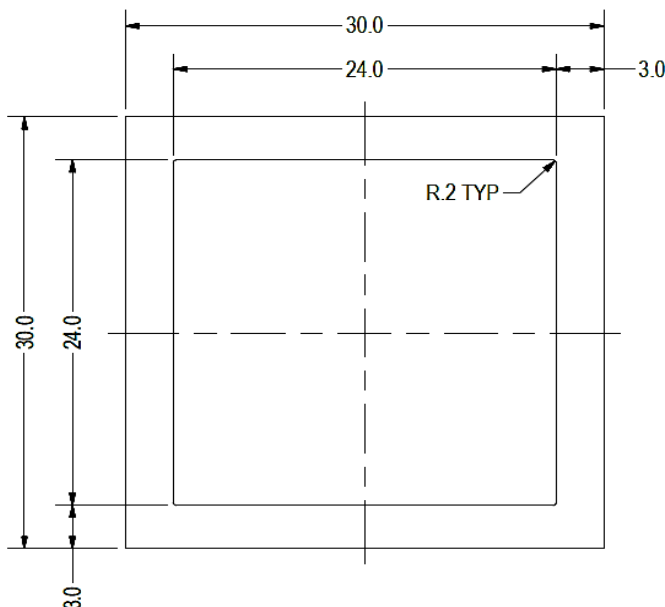
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.764 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

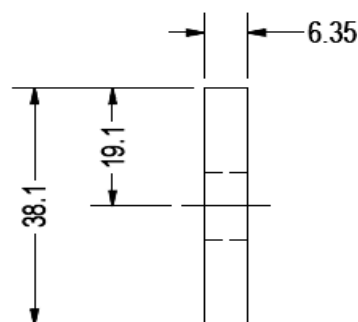
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.695 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	16%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.089 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	7%



A cross brace kit is required every 27 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

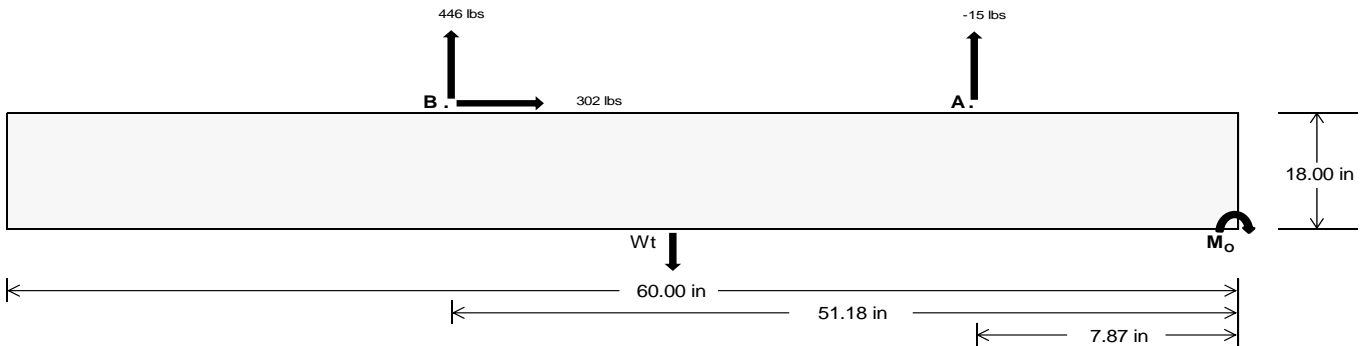
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>26.36</u>	<u>1859.55</u>	k
Compressive Load =	<u>1061.28</u>	<u>1289.94</u>	k
Lateral Load =	<u>2.52</u>	<u>1257.60</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 28174.5$ in-lbs
Resisting Force Required = 939.15 lbs
S.F. = 1.67
Weight Required = 1565.25 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 302.20 lbs
Friction = 0.4
Weight Required = 755.49 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 302.20 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

	Ballast Width			
	22 in	23 in	24 in	25 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$	1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	398 lbs	398 lbs	398 lbs	398 lbs	347 lbs	347 lbs	347 lbs	347 lbs	518 lbs	518 lbs	518 lbs	518 lbs	29 lbs	29 lbs	29 lbs	29 lbs
F_B	259 lbs	259 lbs	259 lbs	259 lbs	570 lbs	570 lbs	570 lbs	570 lbs	594 lbs	594 lbs	594 lbs	594 lbs	-893 lbs	-893 lbs	-893 lbs	-893 lbs
F_V	43 lbs	43 lbs	43 lbs	43 lbs	548 lbs	548 lbs	548 lbs	548 lbs	440 lbs	440 lbs	440 lbs	440 lbs	-604 lbs	-604 lbs	-604 lbs	-604 lbs
P_{total}	2651 lbs	2742 lbs	2832 lbs	2923 lbs	2910 lbs	3001 lbs	3091 lbs	3182 lbs	3106 lbs	3197 lbs	3287 lbs	3378 lbs	333 lbs	387 lbs	441 lbs	496 lbs
M	341 lbs-ft	341 lbs-ft	341 lbs-ft	341 lbs-ft	457 lbs-ft	457 lbs-ft	457 lbs-ft	457 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	724 lbs-ft	724 lbs-ft	724 lbs-ft	724 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.18 ft	1.87 ft	1.64 ft	1.46 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	244.6 psf	243.4 psf	242.3 psf	241.4 psf	257.7 psf	256.0 psf	254.4 psf	252.9 psf	264.8 psf	262.8 psf	260.9 psf	259.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	333.8 psf	328.7 psf	324.1 psf	319.8 psf	377.2 psf	370.3 psf	363.9 psf	358.1 psf	412.9 psf	404.4 psf	396.6 psf	389.4 psf	372.5 psf	213.6 psf	171.0 psf	152.5 psf

Maximum Bearing Pressure = 413 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

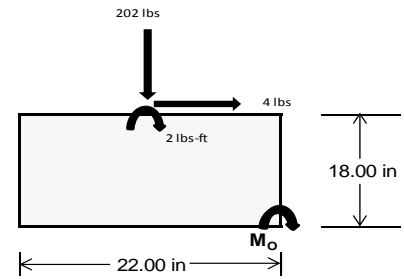
Overturning Check

$M_o = 177.4 \text{ ft-lbs}$
 Resisting Force Required = 193.57 lbs
 S.F. = 1.67
 Weight Required = 322.62 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	69 lbs	168 lbs	65 lbs	202 lbs	551 lbs	198 lbs	20 lbs	49 lbs	19 lbs
F_v	1 lbs	1 lbs	0 lbs	4 lbs	3 lbs	1 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2537 lbs	2636 lbs	2533 lbs	2552 lbs	2901 lbs	2548 lbs	742 lbs	771 lbs	741 lbs
M	1 lbs-ft	1 lbs-ft	0 lbs-ft	8 lbs-ft	5 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	276.3 sqft	287.3 sqft	276.3 sqft	275.5 sqft	314.6 sqft	277.7 sqft	80.8 sqft	84.0 sqft	80.8 sqft
f_{max}	277.2 psf	288.0 psf	276.5 psf	281.3 psf	318.3 psf	278.3 psf	81.1 psf	84.2 psf	80.8 psf



Maximum Bearing Pressure = 318 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.564 k
Allowable Uplift =	1.214 k
Utilization =	<u>46%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.106 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.816 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>

Diagonal Strut

Maximum Axial Load =	0.764 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



Rear Strut

Maximum Axial Load =	1.149 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.089 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.018 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 66.00 \text{ in}$$

$$J = 0.427$$

$$137.652$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 66.00 \text{ in}$$

$$J = 0.427$$

$$149.579$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.837 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.09 \\
 &23.5807 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.09 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.442 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	262.028	2	303.349	2	-.001	15	0	15	0	1	0	1
2		min	-311.868	3	-447.814	3	-.11	3	0	3	0	1	0	1
3	N7	max	.028	3	317.158	1	-.043	15	0	15	0	1	0	1
4		min	-.166	2	13.107	15	-.873	1	-.001	1	0	1	0	1
5	N15	max	.171	3	816.368	1	.476	1	0	1	0	1	0	1
6		min	-1.398	2	29.191	15	-.587	3	0	3	0	1	0	1
7	N16	max	892.026	2	992.261	2	0	10	0	1	0	1	0	1
8		min	-967.385	3	-1430.422	3	-73.765	3	0	3	0	1	0	1
9	N23	max	.028	3	316.987	1	1.937	1	.003	1	0	1	0	1
10		min	-.166	2	13.232	15	.097	15	0	15	0	1	0	1
11	N24	max	262.119	2	306.411	2	74.335	3	0	1	0	1	0	1
12		min	-312.22	3	-446.219	3	.007	10	0	3	0	1	0	1
13	Totals:	max	1414.443	2	2755.753	2	0	3						
14		min	-1591.246	3	-2130.337	3	0	9						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	219.357	1	.678	4	.255	1	0	15	0	15	0	1
2			min	-370.122	3	.159	15	-.039	3	0	1	0	1	0	1
3		2	max	219.492	1	.621	4	.255	1	0	15	0	15	0	15
4			min	-370.02	3	.146	15	-.039	3	0	1	0	1	0	4
5		3	max	219.627	1	.563	4	.255	1	0	15	0	15	0	15
6			min	-369.919	3	.132	15	-.039	3	0	1	0	1	0	4
7		4	max	219.761	1	.506	4	.255	1	0	15	0	15	0	15
8			min	-369.818	3	.119	15	-.039	3	0	1	0	3	0	4
9		5	max	219.896	1	.448	4	.255	1	0	15	0	1	0	15
10			min	-369.717	3	.105	15	-.039	3	0	1	0	3	0	4
11		6	max	220.031	1	.391	4	.255	1	0	15	0	1	0	15
12			min	-369.616	3	.092	15	-.039	3	0	1	0	3	0	4
13		7	max	220.166	1	.333	4	.255	1	0	15	0	1	0	15
14			min	-369.515	3	.078	15	-.039	3	0	1	0	3	0	4
15		8	max	220.301	1	.276	4	.255	1	0	15	0	1	0	15
16			min	-369.413	3	.065	15	-.039	3	0	1	0	3	0	4
17		9	max	220.436	1	.218	4	.255	1	0	15	0	1	0	15
18			min	-369.312	3	.051	15	-.039	3	0	1	0	3	0	4
19		10	max	220.571	1	.161	4	.255	1	0	15	0	1	0	15
20			min	-369.211	3	.037	12	-.039	3	0	1	0	3	0	4
21		11	max	220.705	1	.11	2	.255	1	0	15	0	1	0	15
22			min	-369.11	3	.015	12	-.039	3	0	1	0	3	0	4
23		12	max	220.84	1	.065	2	.255	1	0	15	0	1	0	15
24			min	-369.009	3	-.014	3	-.039	3	0	1	0	3	0	4
25		13	max	220.975	1	.02	2	.255	1	0	15	0	1	0	15
26			min	-368.908	3	-.047	3	-.039	3	0	1	0	3	0	4
27		14	max	221.11	1	-.016	15	.255	1	0	15	0	1	0	15
28			min	-368.807	3	-.081	3	-.039	3	0	1	0	3	0	4
29		15	max	221.245	1	-.03	15	.255	1	0	15	0	1	0	15
30			min	-368.705	3	-.127	4	-.039	3	0	1	0	3	0	4
31		16	max	221.38	1	-.043	15	.255	1	0	15	0	1	0	15
32			min	-368.604	3	-.184	4	-.039	3	0	1	0	3	0	4
33		17	max	221.515	1	-.057	15	.255	1	0	15	0	1	0	15
34			min	-368.503	3	-.242	4	-.039	3	0	1	0	3	0	4
35		18	max	221.65	1	-.07	15	.255	1	0	15	0	1	0	15
36			min	-368.402	3	-.299	4	-.039	3	0	1	0	3	0	4
37		19	max	221.784	1	-.084	15	.255	1	0	15	0	1	0	12



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-368.301	3	-.357	4	-.039	3	0	1	0	3	0	4
39	M3	1	max	219.726	2	1.735	4	-.014	15	0	15	0	1	0	4
40			min	-218.816	3	.408	15	-.287	1	0	1	0	15	0	12
41		2	max	219.656	2	1.559	4	-.014	15	0	15	0	1	0	2
42			min	-218.868	3	.367	15	-.287	1	0	1	0	15	0	3
43		3	max	219.586	2	1.383	4	-.014	15	0	15	0	1	0	2
44			min	-218.921	3	.325	15	-.287	1	0	1	0	15	0	3
45		4	max	219.516	2	1.206	4	-.014	15	0	15	0	1	0	15
46			min	-218.973	3	.284	15	-.287	1	0	1	0	15	0	4
47		5	max	219.446	2	1.03	4	-.014	15	0	15	0	1	0	15
48			min	-219.026	3	.242	15	-.287	1	0	1	0	15	0	4
49		6	max	219.376	2	.854	4	-.014	15	0	15	0	1	0	15
50			min	-219.078	3	.201	15	-.287	1	0	1	0	15	0	4
51		7	max	219.306	2	.677	4	-.014	15	0	15	0	1	0	15
52			min	-219.131	3	.159	15	-.287	1	0	1	0	15	0	4
53		8	max	219.236	2	.501	4	-.014	15	0	15	0	1	0	15
54			min	-219.183	3	.118	15	-.287	1	0	1	0	15	-.001	4
55		9	max	219.166	2	.324	4	-.014	15	0	15	0	1	0	15
56			min	-219.236	3	.076	15	-.287	1	0	1	0	15	-.001	4
57		10	max	219.096	2	.148	4	-.014	15	0	15	0	1	0	15
58			min	-219.288	3	.034	12	-.287	1	0	1	0	15	-.001	4
59		11	max	219.026	2	.005	2	-.014	15	0	15	0	1	0	15
60			min	-219.341	3	-.054	3	-.287	1	0	1	0	15	-.001	4
61		12	max	218.956	2	-.048	15	-.014	15	0	15	0	1	0	15
62			min	-219.393	3	-.205	4	-.287	1	0	1	0	15	-.001	4
63		13	max	218.886	2	-.089	15	-.014	15	0	15	0	1	0	15
64			min	-219.446	3	-.381	4	-.287	1	0	1	0	15	-.001	4
65		14	max	218.816	2	-.131	15	-.014	15	0	15	0	1	0	15
66			min	-219.498	3	-.557	4	-.287	1	0	1	0	15	-.001	4
67		15	max	218.746	2	-.172	15	-.014	15	0	15	0	1	0	15
68			min	-219.551	3	-.734	4	-.287	1	0	1	0	15	0	4
69		16	max	218.676	2	-.214	15	-.014	15	0	15	0	1	0	15
70			min	-219.603	3	-.91	4	-.287	1	0	1	0	10	0	4
71		17	max	218.606	2	-.255	15	-.014	15	0	15	0	15	0	15
72			min	-219.656	3	-1.087	4	-.287	1	0	1	0	2	0	4
73		18	max	218.536	2	-.297	15	-.014	15	0	15	0	15	0	15
74			min	-219.708	3	-1.263	4	-.287	1	0	1	0	1	0	4
75		19	max	218.466	2	-.338	15	-.014	15	0	15	0	15	0	1
76			min	-219.761	3	-1.439	4	-.287	1	0	1	0	1	0	1
77	M4	1	max	315.993	1	0	1	-.043	15	0	1	0	3	0	1
78			min	12.755	15	0	1	-.921	1	0	1	0	2	0	1
79		2	max	316.058	1	0	1	-.043	15	0	1	0	15	0	1
80			min	12.775	15	0	1	-.921	1	0	1	0	1	0	1
81		3	max	316.122	1	0	1	-.043	15	0	1	0	15	0	1
82			min	12.794	15	0	1	-.921	1	0	1	0	1	0	1
83		4	max	316.187	1	0	1	-.043	15	0	1	0	15	0	1
84			min	12.814	15	0	1	-.921	1	0	1	0	1	0	1
85		5	max	316.252	1	0	1	-.043	15	0	1	0	15	0	1
86			min	12.833	15	0	1	-.921	1	0	1	0	1	0	1
87		6	max	316.316	1	0	1	-.043	15	0	1	0	15	0	1
88			min	12.853	15	0	1	-.921	1	0	1	0	1	0	1
89		7	max	316.381	1	0	1	-.043	15	0	1	0	15	0	1
90			min	12.872	15	0	1	-.921	1	0	1	0	1	0	1
91		8	max	316.446	1	0	1	-.043	15	0	1	0	15	0	1
92			min	12.892	15	0	1	-.921	1	0	1	0	1	0	1
93		9	max	316.511	1	0	1	-.043	15	0	1	0	15	0	1
94			min	12.911	15	0	1	-.921	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	316.575	1	0	1	-.043	15	0	1	0	15	0	1
96		min	12.931	15	0	1	-.921	1	0	1	0	1	0	1
97	11	max	316.64	1	0	1	-.043	15	0	1	0	15	0	1
98		min	12.951	15	0	1	-.921	1	0	1	0	1	0	1
99	12	max	316.705	1	0	1	-.043	15	0	1	0	15	0	1
100		min	12.97	15	0	1	-.921	1	0	1	0	1	0	1
101	13	max	316.769	1	0	1	-.043	15	0	1	0	15	0	1
102		min	12.99	15	0	1	-.921	1	0	1	-.001	1	0	1
103	14	max	316.834	1	0	1	-.043	15	0	1	0	15	0	1
104		min	13.009	15	0	1	-.921	1	0	1	-.001	1	0	1
105	15	max	316.899	1	0	1	-.043	15	0	1	0	15	0	1
106		min	13.029	15	0	1	-.921	1	0	1	-.001	1	0	1
107	16	max	316.964	1	0	1	-.043	15	0	1	0	15	0	1
108		min	13.048	15	0	1	-.921	1	0	1	-.001	1	0	1
109	17	max	317.028	1	0	1	-.043	15	0	1	0	15	0	1
110		min	13.068	15	0	1	-.921	1	0	1	-.001	1	0	1
111	18	max	317.093	1	0	1	-.043	15	0	1	0	15	0	1
112		min	13.087	15	0	1	-.921	1	0	1	-.001	1	0	1
113	19	max	317.158	1	0	1	-.043	15	0	1	0	15	0	1
114		min	13.107	15	0	1	-.921	1	0	1	-.001	1	0	1
115	M6	1	max	692.789	1	.68	.061	1	0	3	0	3	0	1
116		min	-1148.578	3	.16	15	-.194	3	0	10	0	9	0	1
117	2	max	692.924	1	.623	4	.061	1	0	3	0	3	0	15
118		min	-1148.477	3	.146	15	-.194	3	0	10	0	9	0	4
119	3	max	693.059	1	.565	4	.061	1	0	3	0	3	0	15
120		min	-1148.376	3	.133	15	-.194	3	0	10	0	10	0	4
121	4	max	693.194	1	.508	4	.061	1	0	3	0	3	0	15
122		min	-1148.275	3	.119	15	-.194	3	0	10	0	10	0	4
123	5	max	693.329	1	.45	4	.061	1	0	3	0	3	0	15
124		min	-1148.174	3	.098	12	-.194	3	0	10	0	10	0	4
125	6	max	693.464	1	.399	2	.061	1	0	3	0	1	0	15
126		min	-1148.073	3	.075	12	-.194	3	0	10	0	10	0	4
127	7	max	693.599	1	.354	2	.061	1	0	3	0	1	0	15
128		min	-1147.971	3	.053	12	-.194	3	0	10	0	10	0	4
129	8	max	693.733	1	.31	2	.061	1	0	3	0	1	0	12
130		min	-1147.87	3	.03	12	-.194	3	0	10	0	3	0	4
131	9	max	693.868	1	.265	2	.061	1	0	3	0	1	0	12
132		min	-1147.769	3	.004	3	-.194	3	0	10	0	3	0	4
133	10	max	694.003	1	.22	2	.061	1	0	3	0	1	0	12
134		min	-1147.668	3	-.029	3	-.194	3	0	10	0	3	0	4
135	11	max	694.138	1	.175	2	.061	1	0	3	0	1	0	12
136		min	-1147.567	3	-.063	3	-.194	3	0	10	0	3	0	2
137	12	max	694.273	1	.13	2	.061	1	0	3	0	1	0	12
138		min	-1147.466	3	-.096	3	-.194	3	0	10	0	3	0	2
139	13	max	694.408	1	.086	2	.061	1	0	3	0	1	0	12
140		min	-1147.365	3	-.13	3	-.194	3	0	10	0	3	0	2
141	14	max	694.543	1	.041	2	.061	1	0	3	0	1	0	12
142		min	-1147.263	3	-.164	3	-.194	3	0	10	0	3	0	2
143	15	max	694.678	1	-.004	2	.061	1	0	3	0	1	0	12
144		min	-1147.162	3	-.197	3	-.194	3	0	10	0	3	0	2
145	16	max	694.812	1	-.043	15	.061	1	0	3	0	1	0	12
146		min	-1147.061	3	-.231	3	-.194	3	0	10	0	3	0	2
147	17	max	694.947	1	-.056	15	.061	1	0	3	0	1	0	3
148		min	-1146.96	3	-.264	3	-.194	3	0	10	0	3	0	2
149	18	max	695.082	1	-.07	15	.061	1	0	3	0	1	0	3
150		min	-1146.859	3	-.298	3	-.194	3	0	10	0	3	0	2
151	19	max	695.217	1	-.083	15	.061	1	0	3	0	1	0	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1146.758	3	-.354	4	-.194	3	0	10	0	3	0	2
153	M7	1	max	763.581	2	1.739	4	.042	3	0	1	0	2	2
154		min	-661.362	3	.409	15	-.017	2	0	3	0	3	0	3
155		2	max	763.511	2	1.563	4	.042	3	0	1	0	2	2
156		min	-661.414	3	.367	15	-.017	2	0	3	0	3	0	3
157		3	max	763.441	2	1.387	4	.042	3	0	1	0	2	2
158		min	-661.467	3	.326	15	-.017	2	0	3	0	3	0	3
159		4	max	763.371	2	1.21	4	.042	3	0	1	0	1	2
160		min	-661.519	3	.284	15	-.017	2	0	3	0	3	0	3
161		5	max	763.301	2	1.034	4	.042	3	0	1	0	1	15
162		min	-661.572	3	.243	15	-.017	2	0	3	0	3	0	3
163		6	max	763.231	2	.857	4	.042	3	0	1	0	1	15
164		min	-661.624	3	.201	15	-.017	2	0	3	0	3	0	4
165		7	max	763.161	2	.681	4	.042	3	0	1	0	1	15
166		min	-661.677	3	.16	15	-.017	2	0	3	0	3	0	4
167		8	max	763.091	2	.505	4	.042	3	0	1	0	1	15
168		min	-661.729	3	.119	15	-.017	2	0	3	0	3	-.001	4
169		9	max	763.021	2	.346	2	.042	3	0	1	0	1	15
170		min	-661.782	3	.054	12	-.017	2	0	3	0	3	-.001	4
171		10	max	762.951	2	.208	2	.042	3	0	1	0	1	15
172		min	-661.834	3	-.03	3	-.017	2	0	3	0	3	-.001	4
173		11	max	762.881	2	.071	2	.042	3	0	1	0	1	15
174		min	-661.887	3	-.133	3	-.017	2	0	3	0	3	-.001	4
175		12	max	762.811	2	-.047	15	.042	3	0	1	0	1	15
176		min	-661.939	3	-.236	3	-.017	2	0	3	0	3	-.001	4
177		13	max	762.741	2	-.089	15	.042	3	0	1	0	1	15
178		min	-661.992	3	-.377	4	-.017	2	0	3	0	3	-.001	4
179		14	max	762.671	2	-.13	15	.042	3	0	1	0	1	15
180		min	-662.044	3	-.554	4	-.017	2	0	3	0	3	-.001	4
181		15	max	762.601	2	-.172	15	.042	3	0	1	0	1	15
182		min	-662.097	3	-.73	4	-.017	2	0	3	0	3	0	4
183		16	max	762.531	2	-.213	15	.042	3	0	1	0	1	15
184		min	-662.149	3	-.906	4	-.017	2	0	3	0	3	0	4
185		17	max	762.461	2	-.255	15	.042	3	0	1	0	1	15
186		min	-662.202	3	-1.083	4	-.017	2	0	3	0	3	0	4
187		18	max	762.391	2	-.296	15	.042	3	0	1	0	1	15
188		min	-662.254	3	-1.259	4	-.017	2	0	3	0	3	0	4
189		19	max	762.321	2	-.338	15	.042	3	0	1	0	1	1
190		min	-662.307	3	-1.435	4	-.017	2	0	3	0	3	0	1
191	M8	1	max	815.203	1	0	1	.549	1	0	1	0	10	1
192		min	28.84	15	0	1	-.597	3	0	1	0	1	0	1
193		2	max	815.268	1	0	1	.549	1	0	1	0	1	1
194		min	28.86	15	0	1	-.597	3	0	1	0	3	0	1
195		3	max	815.332	1	0	1	.549	1	0	1	0	1	1
196		min	28.879	15	0	1	-.597	3	0	1	0	3	0	1
197		4	max	815.397	1	0	1	.549	1	0	1	0	1	1
198		min	28.899	15	0	1	-.597	3	0	1	0	3	0	1
199		5	max	815.462	1	0	1	.549	1	0	1	0	1	1
200		min	28.918	15	0	1	-.597	3	0	1	0	3	0	1
201		6	max	815.526	1	0	1	.549	1	0	1	0	1	1
202		min	28.938	15	0	1	-.597	3	0	1	0	3	0	1
203		7	max	815.591	1	0	1	.549	1	0	1	0	1	1
204		min	28.957	15	0	1	-.597	3	0	1	0	3	0	1
205		8	max	815.656	1	0	1	.549	1	0	1	0	1	1
206		min	28.977	15	0	1	-.597	3	0	1	0	3	0	1
207		9	max	815.721	1	0	1	.549	1	0	1	0	1	1
208		min	28.996	15	0	1	-.597	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	815.785	1	0	1	.549	1	0	1	0	1	0	1
210		min	29.016	15	0	1	-.597	3	0	1	0	3	0	1
211	11	max	815.85	1	0	1	.549	1	0	1	0	1	0	1
212		min	29.035	15	0	1	-.597	3	0	1	0	3	0	1
213	12	max	815.915	1	0	1	.549	1	0	1	0	1	0	1
214		min	29.055	15	0	1	-.597	3	0	1	0	3	0	1
215	13	max	815.979	1	0	1	.549	1	0	1	0	1	0	1
216		min	29.074	15	0	1	-.597	3	0	1	0	3	0	1
217	14	max	816.044	1	0	1	.549	1	0	1	0	1	0	1
218		min	29.094	15	0	1	-.597	3	0	1	0	3	0	1
219	15	max	816.109	1	0	1	.549	1	0	1	0	1	0	1
220		min	29.113	15	0	1	-.597	3	0	1	0	3	0	1
221	16	max	816.174	1	0	1	.549	1	0	1	0	1	0	1
222		min	29.133	15	0	1	-.597	3	0	1	0	3	0	1
223	17	max	816.238	1	0	1	.549	1	0	1	0	1	0	1
224		min	29.152	15	0	1	-.597	3	0	1	0	3	0	1
225	18	max	816.303	1	0	1	.549	1	0	1	0	1	0	1
226		min	29.172	15	0	1	-.597	3	0	1	0	3	0	1
227	19	max	816.368	1	0	1	.549	1	0	1	0	1	0	1
228		min	29.191	15	0	1	-.597	3	0	1	0	3	0	1
229	M10	1	max	221.881	1	.675	.006	3	0	1	0	1	0	1
230		min	-321.661	3	.159	15	-.143	1	0	3	0	3	0	1
231	2	max	222.016	1	.617	4	.006	3	0	1	0	1	0	15
232		min	-321.56	3	.145	15	-.143	1	0	3	0	3	0	4
233	3	max	222.151	1	.56	4	.006	3	0	1	0	1	0	15
234		min	-321.459	3	.132	15	-.143	1	0	3	0	3	0	4
235	4	max	222.286	1	.502	4	.006	3	0	1	0	1	0	15
236		min	-321.358	3	.118	15	-.143	1	0	3	0	3	0	4
237	5	max	222.421	1	.445	4	.006	3	0	1	0	1	0	15
238		min	-321.257	3	.105	15	-.143	1	0	3	0	3	0	4
239	6	max	222.555	1	.387	4	.006	3	0	1	0	1	0	15
240		min	-321.156	3	.091	15	-.143	1	0	3	0	3	0	4
241	7	max	222.69	1	.33	4	.006	3	0	1	0	1	0	15
242		min	-321.055	3	.078	15	-.143	1	0	3	0	3	0	4
243	8	max	222.825	1	.272	4	.006	3	0	1	0	1	0	15
244		min	-320.953	3	.064	15	-.143	1	0	3	0	3	0	4
245	9	max	222.96	1	.215	4	.006	3	0	1	0	1	0	15
246		min	-320.852	3	.051	15	-.143	1	0	3	0	3	0	4
247	10	max	223.095	1	.157	4	.006	3	0	1	0	9	0	15
248		min	-320.751	3	.037	15	-.143	1	0	3	0	3	0	4
249	11	max	223.23	1	.11	2	.006	3	0	1	0	9	0	15
250		min	-320.65	3	.024	15	-.143	1	0	3	0	3	0	4
251	12	max	223.365	1	.065	2	.006	3	0	1	0	15	0	15
252		min	-320.549	3	.005	12	-.143	1	0	3	0	3	0	4
253	13	max	223.499	1	.02	2	.006	3	0	1	0	15	0	15
254		min	-320.448	3	-.028	3	-.143	1	0	3	0	3	0	4
255	14	max	223.634	1	-.017	15	.006	3	0	1	0	15	0	15
256		min	-320.346	3	-.073	4	-.143	1	0	3	0	3	0	4
257	15	max	223.769	1	-.03	15	.006	3	0	1	0	15	0	15
258		min	-320.245	3	-.13	4	-.143	1	0	3	0	3	0	4
259	16	max	223.904	1	-.044	15	.006	3	0	1	0	15	0	15
260		min	-320.144	3	-.188	4	-.143	1	0	3	0	3	0	4
261	17	max	224.039	1	-.057	15	.006	3	0	1	0	15	0	15
262		min	-320.043	3	-.245	4	-.143	1	0	3	0	1	0	4
263	18	max	224.174	1	-.071	15	.006	3	0	1	0	15	0	15
264		min	-319.942	3	-.303	4	-.143	1	0	3	0	1	0	4
265	19	max	224.309	1	-.084	15	.006	3	0	1	0	15	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-319.841	3	-.36	4	-.143	1	0	3	0	1	0	4
267	M11	1	max	219.268	2	1.738	4	.308	1	0	1	0	3	0	4
268			min	-219.516	3	.409	15	-.041	3	0	15	0	1	0	12
269		2	max	219.198	2	1.562	4	.308	1	0	1	0	3	0	2
270			min	-219.568	3	.367	15	-.041	3	0	15	0	1	0	3
271		3	max	219.128	2	1.386	4	.308	1	0	1	0	3	0	2
272			min	-219.621	3	.326	15	-.041	3	0	15	0	1	0	3
273		4	max	219.058	2	1.209	4	.308	1	0	1	0	3	0	15
274			min	-219.673	3	.284	15	-.041	3	0	15	0	1	0	3
275		5	max	218.988	2	1.033	4	.308	1	0	1	0	3	0	15
276			min	-219.726	3	.243	15	-.041	3	0	15	0	1	0	4
277		6	max	218.918	2	.857	4	.308	1	0	1	0	3	0	15
278			min	-219.778	3	.201	15	-.041	3	0	15	0	1	0	4
279		7	max	218.848	2	.68	4	.308	1	0	1	0	3	0	15
280			min	-219.831	3	.16	15	-.041	3	0	15	0	1	0	4
281		8	max	218.778	2	.504	4	.308	1	0	1	0	3	0	15
282			min	-219.883	3	.118	15	-.041	3	0	15	0	1	-.001	4
283		9	max	218.708	2	.327	4	.308	1	0	1	0	3	0	15
284			min	-219.936	3	.077	15	-.041	3	0	15	0	1	-.001	4
285		10	max	218.638	2	.151	4	.308	1	0	1	0	3	0	15
286			min	-219.988	3	.027	12	-.041	3	0	15	0	1	-.001	4
287		11	max	218.568	2	.005	2	.308	1	0	1	0	3	0	15
288			min	-220.041	3	-.066	3	-.041	3	0	15	0	1	-.001	4
289		12	max	218.498	2	-.047	15	.308	1	0	1	0	3	0	15
290			min	-220.093	3	-.202	4	-.041	3	0	15	0	1	-.001	4
291		13	max	218.428	2	-.089	15	.308	1	0	1	0	3	0	15
292			min	-220.146	3	-.378	4	-.041	3	0	15	0	1	-.001	4
293		14	max	218.358	2	-.13	15	.308	1	0	1	0	3	0	15
294			min	-220.198	3	-.554	4	-.041	3	0	15	0	1	-.001	4
295		15	max	218.288	2	-.172	15	.308	1	0	1	0	3	0	15
296			min	-220.251	3	-.731	4	-.041	3	0	15	0	1	0	4
297		16	max	218.218	2	-.213	15	.308	1	0	1	0	3	0	15
298			min	-220.303	3	-.907	4	-.041	3	0	15	0	2	0	4
299		17	max	218.148	2	-.255	15	.308	1	0	1	0	3	0	15
300			min	-220.356	3	-1.084	4	-.041	3	0	15	0	10	0	4
301		18	max	218.078	2	-.296	15	.308	1	0	1	0	3	0	15
302			min	-220.408	3	-1.26	4	-.041	3	0	15	0	15	0	4
303		19	max	218.008	2	-.338	15	.308	1	0	1	0	1	0	1
304			min	-220.461	3	-1.436	4	-.041	3	0	15	0	15	0	1
305	M12	1	max	315.822	1	0	1	2.043	1	0	1	0	2	0	1
306			min	12.881	15	0	1	.098	15	0	1	0	3	0	1
307		2	max	315.887	1	0	1	2.043	1	0	1	0	1	0	1
308			min	12.9	15	0	1	.098	15	0	1	0	15	0	1
309		3	max	315.952	1	0	1	2.043	1	0	1	0	1	0	1
310			min	12.92	15	0	1	.098	15	0	1	0	15	0	1
311		4	max	316.016	1	0	1	2.043	1	0	1	0	1	0	1
312			min	12.939	15	0	1	.098	15	0	1	0	15	0	1
313		5	max	316.081	1	0	1	2.043	1	0	1	0	1	0	1
314			min	12.959	15	0	1	.098	15	0	1	0	15	0	1
315		6	max	316.146	1	0	1	2.043	1	0	1	0	1	0	1
316			min	12.978	15	0	1	.098	15	0	1	0	15	0	1
317		7	max	316.21	1	0	1	2.043	1	0	1	.001	1	0	1
318			min	12.998	15	0	1	.098	15	0	1	0	15	0	1
319		8	max	316.275	1	0	1	2.043	1	0	1	.001	1	0	1
320			min	13.017	15	0	1	.098	15	0	1	0	15	0	1
321		9	max	316.34	1	0	1	2.043	1	0	1	.001	1	0	1
322			min	13.037	15	0	1	.098	15	0	1	0	15	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323		10	max	316.404	1	0	1	2.043	1	0	1	.002	1	0	1
324			min	13.056	15	0	1	.098	15	0	1	0	15	0	1
325		11	max	316.469	1	0	1	2.043	1	0	1	.002	1	0	1
326			min	13.076	15	0	1	.098	15	0	1	0	15	0	1
327		12	max	316.534	1	0	1	2.043	1	0	1	.002	1	0	1
328			min	13.095	15	0	1	.098	15	0	1	0	15	0	1
329		13	max	316.599	1	0	1	2.043	1	0	1	.002	1	0	1
330			min	13.115	15	0	1	.098	15	0	1	0	15	0	1
331		14	max	316.663	1	0	1	2.043	1	0	1	.002	1	0	1
332			min	13.135	15	0	1	.098	15	0	1	0	15	0	1
333		15	max	316.728	1	0	1	2.043	1	0	1	.003	1	0	1
334			min	13.154	15	0	1	.098	15	0	1	0	15	0	1
335		16	max	316.793	1	0	1	2.043	1	0	1	.003	1	0	1
336			min	13.174	15	0	1	.098	15	0	1	0	15	0	1
337		17	max	316.857	1	0	1	2.043	1	0	1	.003	1	0	1
338			min	13.193	15	0	1	.098	15	0	1	0	15	0	1
339		18	max	316.922	1	0	1	2.043	1	0	1	.003	1	0	1
340			min	13.213	15	0	1	.098	15	0	1	0	15	0	1
341		19	max	316.987	1	0	1	2.043	1	0	1	.003	1	0	1
342			min	13.232	15	0	1	.098	15	0	1	0	15	0	1
343	M1	1	max	110.844	1	345.146	3	-1.907	15	0	2	.079	1	.013	2
344			min	5.096	15	-232.706	2	-40.253	1	0	3	.004	15	-.015	3
345		2	max	111.004	1	344.975	3	-1.907	15	0	2	.07	1	.064	2
346			min	5.144	15	-232.934	2	-40.253	1	0	3	.003	15	-.09	3
347		3	max	117.574	3	5.331	9	-1.895	15	0	12	.061	1	.114	2
348			min	-19.038	10	-30.964	2	-40.162	1	0	1	.003	15	-.163	3
349		4	max	117.694	3	5.14	9	-1.895	15	0	12	.052	1	.12	2
350			min	-18.904	10	-31.192	2	-40.162	1	0	1	.002	15	-.161	3
351		5	max	117.814	3	4.949	9	-1.895	15	0	12	.044	1	.127	2
352			min	-18.771	10	-31.421	2	-40.162	1	0	1	.002	15	-.159	3
353		6	max	117.934	3	4.759	9	-1.895	15	0	12	.035	1	.134	2
354			min	-18.637	10	-31.65	2	-40.162	1	0	1	.002	15	-.157	3
355		7	max	118.054	3	4.568	9	-1.895	15	0	12	.026	1	.141	2
356			min	-18.504	10	-31.878	2	-40.162	1	0	1	.001	15	-.154	3
357		8	max	118.174	3	4.378	9	-1.895	15	0	12	.018	1	.148	2
358			min	-18.37	10	-32.107	2	-40.162	1	0	1	0	15	-.152	3
359		9	max	118.294	3	4.187	9	-1.895	15	0	12	.009	1	.155	2
360			min	-18.237	10	-32.336	2	-40.162	1	0	1	0	15	-.15	3
361		10	max	118.414	3	3.996	9	-1.895	15	0	12	.002	3	.162	2
362			min	-18.103	10	-32.565	2	-40.162	1	0	1	0	10	-.148	3
363		11	max	118.535	3	3.806	9	-1.895	15	0	12	0	3	.169	2
364			min	-17.97	10	-32.793	2	-40.162	1	0	1	-.009	1	-.145	3
365		12	max	118.655	3	3.615	9	-1.895	15	0	12	0	12	.176	2
366			min	-17.836	10	-33.022	2	-40.162	1	0	1	-.017	1	-.143	3
367		13	max	118.775	3	3.425	9	-1.895	15	0	12	-.001	15	.183	2
368			min	-17.703	10	-33.251	2	-40.162	1	0	1	-.026	1	-.14	3
369		14	max	118.895	3	3.234	9	-1.895	15	0	12	-.002	15	.19	2
370			min	-17.569	10	-33.48	2	-40.162	1	0	1	-.035	1	-.138	3
371		15	max	119.015	3	3.043	9	-1.895	15	0	12	-.002	15	.198	2
372			min	-17.436	10	-33.708	2	-40.162	1	0	1	-.043	1	-.135	3
373		16	max	90.328	2	158.822	2	-1.909	15	0	1	-.002	15	.203	2
374			min	2.081	15	-206.515	3	-40.403	1	0	12	-.052	1	-.131	3
375		17	max	90.488	2	158.593	2	-1.909	15	0	1	-.003	15	.169	2
376			min	2.13	15	-206.687	3	-40.403	1	0	12	-.061	1	-.086	3
377		18	max	-5.138	15	350.356	2	-1.952	15	0	3	-.003	15	.094	2
378			min	-110.993	1	-170.138	3	-41.405	1	0	2	-.07	1	-.049	3
379		19	max	-5.09	15	350.128	2	-1.952	15	0	3	-.004	15	.018	2



Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	118.297	3	3.974	9	39.218	1	0	1	.01	3	.162	2
438			min	-17.628	10	-32.574	2	-1.23	3	0	12	0	1	-.148	3
439		11	max	118.417	3	3.783	9	39.218	1	0	1	.01	3	.169	2
440			min	-17.495	10	-32.802	2	-1.23	3	0	12	0	15	-.145	3
441		12	max	118.538	3	3.593	9	39.218	1	0	1	.017	1	.176	2
442			min	-17.361	10	-33.031	2	-1.23	3	0	12	0	15	-.143	3
443		13	max	118.658	3	3.402	9	39.218	1	0	1	.025	1	.183	2
444			min	-17.228	10	-33.26	2	-1.23	3	0	12	.001	15	-.14	3
445		14	max	118.778	3	3.211	9	39.218	1	0	1	.034	1	.19	2
446			min	-17.095	10	-33.489	2	-1.23	3	0	12	.002	15	-.138	3
447		15	max	118.898	3	3.021	9	39.218	1	0	1	.043	1	.198	2
448			min	-16.961	10	-33.717	2	-1.23	3	0	12	.002	15	-.135	3
449		16	max	90.646	2	158.41	2	39.475	1	0	15	.051	1	.203	2
450			min	2.172	15	-207.062	3	-1.273	3	0	1	.002	15	-.131	3
451		17	max	90.806	2	158.181	2	39.475	1	0	15	.06	1	.169	2
452			min	2.221	15	-207.234	3	-1.273	3	0	1	.003	15	-.086	3
453		18	max	-5.119	15	350.357	2	41.552	1	0	2	.069	1	.094	2
454			min	-110.614	1	-170.131	3	-.784	3	0	3	.003	15	-.049	3
455		19	max	-5.071	15	350.128	2	41.552	1	0	2	.078	1	.018	2
456			min	-110.453	1	-170.302	3	-.784	3	0	3	.004	15	-.012	3
457	M13	1	max	72.18	3	232.544	2	-5.073	15	.013	2	.078	1	0	2
458			min	2.061	15	-345.102	3	-110.449	1	-.015	3	.004	15	0	3
459		2	max	72.18	3	165.375	2	-3.865	15	.013	2	.019	1	.18	3
460			min	2.061	15	-244.877	3	-83.871	1	-.015	3	0	10	-.122	2
461		3	max	72.18	3	98.207	2	-2.657	15	.013	2	.008	3	.299	3
462			min	2.061	15	-144.652	3	-57.294	1	-.015	3	-.024	1	-.202	2
463		4	max	72.18	3	31.038	2	-1.449	15	.013	2	.004	3	.357	3
464			min	2.061	15	-44.428	3	-30.716	1	-.015	3	-.051	1	-.242	2
465		5	max	72.18	3	55.797	3	.754	10	.013	2	.001	3	.354	3
466			min	2.061	15	-36.131	2	-4.139	1	-.015	3	-.062	1	-.24	2
467		6	max	72.18	3	156.022	3	22.438	1	.013	2	0	3	.289	3
468			min	2.061	15	-103.299	2	-1.794	3	-.015	3	-.056	1	-.197	2
469		7	max	72.18	3	256.247	3	49.016	1	.013	2	0	12	.163	3
470			min	2.061	15	-170.468	2	-.025	3	-.015	3	-.035	1	-.114	2
471		8	max	72.18	3	356.471	3	75.593	1	.013	2	.005	2	.013	1
472			min	2.061	15	-237.637	2	1.296	12	-.015	3	0	3	-.024	3
473		9	max	72.18	3	456.696	3	102.171	1	.013	2	.058	1	.177	2
474			min	2.061	15	-304.805	2	2.474	12	-.015	3	0	12	-.273	3
475		10	max	72.18	3	556.921	3	128.748	1	.013	2	.128	1	.383	2
476			min	2.061	15	-371.974	2	3.653	12	-.015	3	-.005	3	-.583	3
477		11	max	40.359	1	304.805	2	-2.127	12	.015	3	.057	1	.177	2
478			min	1.907	15	-456.696	3	-101.783	1	-.013	2	-.008	3	-.273	3
479		12	max	40.359	1	237.637	2	-.948	12	.015	3	.004	2	.013	1
480			min	1.907	15	-356.471	3	-75.206	1	-.013	2	-.009	3	-.024	3
481		13	max	40.359	1	170.468	2	.576	3	.015	3	-.002	15	.163	3
482			min	1.907	15	-256.247	3	-48.629	1	-.013	2	-.035	1	-.114	2
483		14	max	40.359	1	103.299	2	2.344	3	.015	3	-.003	15	.289	3
484			min	1.907	15	-156.022	3	-22.051	1	-.013	2	-.057	1	-.197	2
485		15	max	40.359	1	36.131	2	4.526	1	.015	3	-.003	15	.354	3
486			min	1.907	15	-55.797	3	-.754	10	-.013	2	-.062	1	-.24	2
487		16	max	40.359	1	44.428	3	31.104	1	.015	3	-.002	12	.357	3
488			min	1.907	15	-31.038	2	1.471	15	-.013	2	-.051	1	-.242	2
489		17	max	40.359	1	144.652	3	57.681	1	.015	3	.001	3	.299	3
490			min	1.907	15	-98.207	2	2.679	15	-.013	2	-.024	1	-.202	2
491		18	max	40.359	1	244.877	3	84.258	1	.015	3	.019	1	.18	3
492			min	1.907	15	-165.375	2	3.887	15	-.013	2	0	10	-.122	2
493		19	max	40.359	1	345.102	3	110.836	1	.015	3	.079	1	0	2



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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	1.907	15	-232.544	2	5.096	15	-.013	2	.004	15	0	3
495		max	.788	3	350.334	2	-5.071	15	.012	3	.078	1	0	2
496		min	-41.442	1	-170.338	3	-110.462	1	-.018	2	.004	15	0	3
497		2 max	.788	3	248.916	2	-3.863	15	.012	3	.019	1	.089	3
498		min	-41.442	1	-121.487	3	-83.884	1	-.018	2	0	10	-.183	2
499		3 max	.788	3	147.498	2	-2.655	15	.012	3	0	12	.148	3
500		min	-41.442	1	-72.637	3	-57.307	1	-.018	2	-.024	1	-.304	2
501		4 max	.788	3	46.079	2	-1.447	15	.012	3	-.002	15	.178	3
502		min	-41.442	1	-23.787	3	-30.73	1	-.018	2	-.051	1	-.363	2
503		5 max	.788	3	25.064	3	.735	10	.012	3	-.003	15	.178	3
504		min	-41.442	1	-55.339	2	-4.152	1	-.018	2	-.062	1	-.361	2
505		6 max	.788	3	73.914	3	22.425	1	.012	3	-.003	15	.147	3
506		min	-41.442	1	-156.757	2	-.571	3	-.018	2	-.056	1	-.296	2
507		7 max	.788	3	122.764	3	49.003	1	.012	3	-.002	15	.087	3
508		min	-41.442	1	-258.176	2	.882	12	-.018	2	-.035	1	-.169	2
509		8 max	.788	3	171.615	3	75.58	1	.012	3	.004	2	.02	2
510		min	-41.442	1	-359.594	2	2.06	12	-.018	2	-.006	3	-.003	3
511		9 max	.788	3	220.465	3	102.157	1	.012	3	.058	1	.271	2
512		min	-41.442	1	-461.012	2	3.239	12	-.018	2	-.003	3	-.123	3
513		10 max	-1.952	15	-9.585	15	128.735	1	0	15	.128	1	.583	2
514		min	-41.442	1	-562.43	2	-7.325	3	-.018	2	.004	12	-.272	3
515		11 max	-1.952	15	461.012	2	-3.751	12	.018	2	.057	1	.271	2
516		min	-41.301	1	-220.465	3	-101.778	1	-.012	3	.001	12	-.123	3
517		12 max	-1.952	15	359.594	2	-2.572	12	.018	2	.004	2	.02	2
518		min	-41.301	1	-171.615	3	-75.201	1	-.012	3	0	3	-.003	3
519		13 max	-1.952	15	258.175	2	-1.393	12	.018	2	-.002	15	.087	3
520		min	-41.301	1	-122.764	3	-48.624	1	-.012	3	-.035	1	-.169	2
521		14 max	-1.952	15	156.757	2	-.215	12	.018	2	-.002	12	.147	3
522		min	-41.301	1	-73.914	3	-22.046	1	-.012	3	-.056	1	-.296	2
523		15 max	-1.952	15	55.339	2	4.531	1	.018	2	-.002	12	.178	3
524		min	-41.301	1	-25.064	3	-.735	10	-.012	3	-.062	1	-.361	2
525		16 max	-1.952	15	23.787	3	31.109	1	.018	2	-.001	12	.178	3
526		min	-41.301	1	-46.079	2	1.466	15	-.012	3	-.051	1	-.363	2
527		17 max	-1.952	15	72.637	3	57.686	1	.018	2	0	3	.148	3
528		min	-41.301	1	-147.498	2	2.674	15	-.012	3	-.024	1	-.304	2
529		18 max	-1.952	15	121.487	3	84.264	1	.018	2	.02	1	.089	3
530		min	-41.301	1	-248.916	2	3.882	15	-.012	3	0	10	-.183	2
531		19 max	-1.952	15	170.338	3	110.841	1	.018	2	.079	1	0	2
532		min	-41.301	1	-350.334	2	5.09	15	-.012	3	.004	15	0	3
533	M15	1 max	.399	13	1.945	4	.073	3	0	9	0	9	0	1
534		min	-87.721	3	0	1	-.017	9	0	3	0	3	0	1
535		2 max	.295	13	1.728	4	.073	3	0	9	0	9	0	1
536		min	-87.797	3	0	1	-.017	9	0	3	0	3	0	4
537		3 max	.191	13	1.512	4	.073	3	0	9	0	9	0	1
538		min	-87.873	3	0	1	-.017	9	0	3	0	3	-.001	4
539		4 max	.087	13	1.296	4	.073	3	0	9	0	9	0	1
540		min	-87.948	3	0	1	-.017	9	0	3	0	3	-.002	4
541		5 max	0	1	1.08	4	.073	3	0	9	0	9	0	1
542		min	-88.024	3	0	1	-.017	9	0	3	0	3	-.002	4
543		6 max	0	1	.864	4	.073	3	0	9	0	9	0	1
544		min	-88.099	3	0	1	-.017	9	0	3	0	3	-.003	4
545		7 max	0	1	.648	4	.073	3	0	9	0	3	0	1
546		min	-88.175	3	0	1	-.017	9	0	3	0	9	-.003	4
547		8 max	0	1	.432	4	.073	3	0	9	0	3	0	1
548		min	-88.25	3	0	1	-.017	9	0	3	0	9	-.003	4
549		9 max	0	1	.216	4	.073	3	0	9	0	3	0	1
550		min	-88.326	3	0	1	-.017	9	0	3	0	9	-.003	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.073	3	0	9	0	3	0	1
552		min	-88.401	3	0	1	-.017	9	0	3	0	9	-.003	4
553	11	max	0	1	0	1	.073	3	0	9	0	3	0	1
554		min	-88.477	3	-.216	4	-.017	9	0	3	0	9	-.003	4
555	12	max	0	1	0	1	.073	3	0	9	0	3	0	1
556		min	-88.552	3	-.432	4	-.017	9	0	3	0	9	-.003	4
557	13	max	0	1	0	1	.073	3	0	9	0	3	0	1
558		min	-88.628	3	-.648	4	-.017	9	0	3	0	9	-.003	4
559	14	max	0	1	0	1	.073	3	0	9	0	3	0	1
560		min	-88.703	3	-.864	4	-.017	9	0	3	0	9	-.003	4
561	15	max	0	1	0	1	.073	3	0	9	0	3	0	1
562		min	-88.779	3	-1.08	4	-.017	9	0	3	0	9	-.002	4
563	16	max	0	1	0	1	.073	3	0	9	0	3	0	1
564		min	-88.854	3	-1.296	4	-.017	9	0	3	0	9	-.002	4
565	17	max	0	1	0	1	.073	3	0	9	0	3	0	1
566		min	-88.93	3	-1.512	4	-.017	9	0	3	0	9	-.001	4
567	18	max	0	1	0	1	.073	3	0	9	0	3	0	1
568		min	-89.006	3	-1.728	4	-.017	9	0	3	0	9	0	4
569	19	max	0	1	0	1	.073	3	0	9	0	3	0	1
570		min	-89.081	3	-1.945	4	-.017	9	0	3	0	9	0	1
571	M16A	1	max	0	10	1.945	.029	2	0	3	0	3	0	1
572		min	-87.971	3	0	10	-.034	3	0	2	0	2	0	1
573	2	max	0	10	1.728	4	.029	2	0	3	0	3	0	10
574		min	-87.896	3	0	10	-.034	3	0	2	0	2	0	4
575	3	max	0	10	1.512	4	.029	2	0	3	0	3	0	10
576		min	-87.82	3	0	10	-.034	3	0	2	0	1	-.001	4
577	4	max	0	10	1.296	4	.029	2	0	3	0	3	0	10
578		min	-87.745	3	0	10	-.034	3	0	2	0	1	-.002	4
579	5	max	0	10	1.08	4	.029	2	0	3	0	3	0	10
580		min	-87.669	3	0	10	-.034	3	0	2	0	1	-.002	4
581	6	max	0	10	.864	4	.029	2	0	3	0	3	0	10
582		min	-87.594	3	0	10	-.034	3	0	2	0	1	-.003	4
583	7	max	0	10	.648	4	.029	2	0	3	0	3	0	10
584		min	-87.518	3	0	10	-.034	3	0	2	0	1	-.003	4
585	8	max	0	10	.432	4	.029	2	0	3	0	3	0	10
586		min	-87.443	3	0	10	-.034	3	0	2	0	1	-.003	4
587	9	max	0	10	.216	4	.029	2	0	3	0	3	0	10
588		min	-87.367	3	0	10	-.034	3	0	2	0	1	-.003	4
589	10	max	0	10	0	1	.029	2	0	3	0	3	0	10
590		min	-87.291	3	0	1	-.034	3	0	2	0	1	-.003	4
591	11	max	0	10	0	10	.029	2	0	3	0	3	0	10
592		min	-87.216	3	-.216	4	-.034	3	0	2	0	1	-.003	4
593	12	max	.076	2	0	10	.029	2	0	3	0	3	0	10
594		min	-87.14	3	-.432	4	-.034	3	0	2	0	1	-.003	4
595	13	max	.177	2	0	10	.029	2	0	3	0	2	0	10
596		min	-87.065	3	-.648	4	-.034	3	0	2	0	4	-.003	4
597	14	max	.277	2	0	10	.029	2	0	3	0	2	0	10
598		min	-86.989	3	-.864	4	-.034	3	0	2	0	3	-.003	4
599	15	max	.378	2	0	10	.029	2	0	3	0	2	0	10
600		min	-86.914	3	-1.08	4	-.034	3	0	2	0	3	-.002	4
601	16	max	.479	2	0	10	.029	2	0	3	0	2	0	10
602		min	-86.838	3	-1.296	4	-.034	3	0	2	0	3	-.002	4
603	17	max	.579	2	0	10	.029	2	0	3	0	2	0	10
604		min	-86.763	3	-1.512	4	-.034	3	0	2	0	3	-.001	4
605	18	max	.68	2	0	10	.029	2	0	3	0	2	0	10
606		min	-86.687	3	-1.728	4	-.034	3	0	2	0	3	0	4
607	19	max	.781	2	0	10	.029	2	0	3	0	2	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-86.612	3	-1.945	4	-.034	3	0	2	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.012	2	.008	1	-3.11e-5	15	NC	3	NC	2	
2			min	-.004	3	-.012	3	-.002	3	-6.582e-4	1	3653.922	2	5345.344	1	
3			2	max	.002	1	.011	2	.007	1	-2.974e-5	15	NC	3	NC	2
4			min	-.004	3	-.011	3	-.002	3	-6.293e-4	1	3986.715	2	5741.029	1	
5			3	max	.002	1	.01	2	.007	1	-2.838e-5	15	NC	3	NC	2
6			min	-.003	3	-.011	3	-.001	3	-6.004e-4	1	4382.124	2	6209.563	1	
7			4	max	.002	1	.009	2	.006	1	-2.701e-5	15	NC	3	NC	2
8			min	-.003	3	-.01	3	-.001	3	-5.715e-4	1	4854.966	2	6768.102	1	
9			5	max	.002	1	.008	2	.006	1	-2.565e-5	15	NC	1	NC	2
10			min	-.003	3	-.01	3	-.001	3	-5.426e-4	1	5424.891	2	7439.558	1	
11		6	max	.002	1	.007	2	.005	1	-2.429e-5	15	NC	1	NC	2	
12		min	-.003	3	-.009	3	-.001	3	-5.136e-4	1	6118.362	2	8255.06	1		
13		7	max	.002	1	.006	2	.005	1	-2.292e-5	15	NC	1	NC	2	
14		min	-.003	3	-.009	3	0	3	-4.847e-4	1	6971.671	2	9257.763	1		
15		8	max	.001	1	.005	2	.004	1	-2.156e-5	15	NC	1	NC	1	
16		min	-.002	3	-.008	3	0	3	-4.558e-4	1	8035.654	2	NC	1		
17		9	max	.001	1	.005	2	.004	1	-2.02e-5	15	NC	1	NC	1	
18		min	-.002	3	-.008	3	0	3	-4.269e-4	1	9383.322	2	NC	1		
19		10	max	.001	1	.004	2	.003	1	-1.883e-5	15	NC	1	NC	1	
20		min	-.002	3	-.007	3	0	3	-3.98e-4	1	NC	1	NC	1		
21		11	max	.001	1	.003	2	.003	1	-1.747e-5	15	NC	1	NC	1	
22		min	-.002	3	-.006	3	0	3	-3.691e-4	1	NC	1	NC	1		
23		12	max	0	1	.003	2	.002	1	-1.61e-5	15	NC	1	NC	1	
24		min	-.002	3	-.006	3	0	3	-3.402e-4	1	NC	1	NC	1		
25		13	max	0	1	.002	2	.002	1	-1.474e-5	15	NC	1	NC	1	
26		min	-.001	3	-.005	3	0	3	-3.113e-4	1	NC	1	NC	1		
27		14	max	0	1	.002	2	.001	1	-1.338e-5	15	NC	1	NC	1	
28		min	-.001	3	-.004	3	0	3	-2.824e-4	1	NC	1	NC	1		
29		15	max	0	1	.001	2	0	1	-1.201e-5	15	NC	1	NC	1	
30		min	0	3	-.003	3	0	3	-2.535e-4	1	NC	1	NC	1		
31		16	max	0	1	0	2	0	1	-1.065e-5	15	NC	1	NC	1	
32		min	0	3	-.003	3	0	3	-2.246e-4	1	NC	1	NC	1		
33		17	max	0	1	0	2	0	1	-9.287e-6	15	NC	1	NC	1	
34		min	0	3	-.002	3	0	3	-1.957e-4	1	NC	1	NC	1		
35		18	max	0	1	0	2	0	1	-7.924e-6	15	NC	1	NC	1	
36		min	0	3	0	3	0	3	-1.668e-4	1	NC	1	NC	1		
37		19	max	0	1	0	1	0	1	-6.56e-6	15	NC	1	NC	1	
38		min	0	1	0	1	0	1	-1.379e-4	1	NC	1	NC	1		
39	M3	1	max	0	1	0	1	0	1	6.603e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.141e-6	15	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	8.014e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	3.792e-6	15	NC	1	NC	1	
43			3	max	0	3	0	2	0	12	9.426e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	4.443e-6	15	NC	1	NC	1	
45			4	max	0	3	0	2	0	12	1.084e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	5.095e-6	15	NC	1	NC	1	
47			5	max	0	3	0	2	0	3	1.225e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	0	1	5.746e-6	15	NC	1	NC	1	
49			6	max	0	3	0	2	0	3	1.366e-4	1	NC	1	NC	1
50			min	0	2	-.005	3	0	1	6.397e-6	15	NC	1	NC	1	
51		7	max	0	3	.001	2	0	3	1.507e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.006	3	0	1	7.048e-6	15	NC	1	NC	1
53		8	max	0	3	.001	2	0	3	1.648e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	7.699e-6	15	NC	1	NC	1
55		9	max	.001	3	.002	2	0	3	1.79e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	9	8.35e-6	15	NC	1	NC	1
57		10	max	.001	3	.002	2	0	2	1.931e-4	1	NC	1	NC	1
58			min	-.001	2	-.008	3	0	15	9.002e-6	15	NC	1	NC	1
59		11	max	.001	3	.003	2	0	1	2.072e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	15	9.653e-6	15	NC	1	NC	1
61		12	max	.002	3	.004	2	0	1	2.213e-4	1	NC	1	NC	1
62			min	-.002	2	-.009	3	0	15	1.03e-5	15	NC	1	NC	1
63		13	max	.002	3	.004	2	.001	1	2.354e-4	1	NC	1	NC	1
64			min	-.002	2	-.009	3	0	15	1.096e-5	15	NC	1	NC	1
65		14	max	.002	3	.005	2	.002	1	2.495e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	15	1.161e-5	15	8827.768	2	NC	1
67		15	max	.002	3	.006	2	.002	1	2.637e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	15	1.226e-5	15	7473.745	2	NC	1
69		16	max	.002	3	.007	2	.003	1	2.778e-4	1	NC	1	NC	1
70			min	-.002	2	-.01	3	0	15	1.291e-5	15	6415.955	2	NC	1
71		17	max	.002	3	.008	2	.003	1	2.919e-4	1	NC	1	NC	1
72			min	-.002	2	-.01	3	0	15	1.356e-5	15	5581.309	2	NC	1
73		18	max	.002	3	.009	2	.004	1	3.06e-4	1	NC	3	NC	1
74			min	-.002	2	-.01	3	0	15	1.421e-5	15	4917.086	2	NC	1
75		19	max	.002	3	.011	2	.004	1	3.201e-4	1	NC	3	NC	1
76			min	-.002	2	-.01	3	0	15	1.486e-5	15	4384.952	2	NC	1
77	M4	1	max	.002	1	.014	2	0	15	-1.615e-5	12	NC	1	NC	2
78			min	0	15	-.012	3	-.003	1	-5.231e-4	1	NC	1	6556.287	1
79		2	max	.001	1	.013	2	0	15	-1.615e-5	12	NC	1	NC	2
80			min	0	15	-.011	3	-.003	1	-5.231e-4	1	NC	1	7150.899	1
81		3	max	.001	1	.012	2	0	15	-1.615e-5	12	NC	1	NC	2
82			min	0	15	-.011	3	-.002	1	-5.231e-4	1	NC	1	7858.65	1
83		4	max	.001	1	.011	2	0	15	-1.615e-5	12	NC	1	NC	2
84			min	0	15	-.01	3	-.002	1	-5.231e-4	1	NC	1	8709.369	1
85		5	max	.001	1	.011	2	0	15	-1.615e-5	12	NC	1	NC	2
86			min	0	15	-.009	3	-.002	1	-5.231e-4	1	NC	1	9743.692	1
87		6	max	.001	1	.01	2	0	15	-1.615e-5	12	NC	1	NC	1
88			min	0	15	-.009	3	-.002	1	-5.231e-4	1	NC	1	NC	1
89		7	max	.001	1	.009	2	0	15	-1.615e-5	12	NC	1	NC	1
90			min	0	15	-.008	3	-.002	1	-5.231e-4	1	NC	1	NC	1
91		8	max	0	1	.008	2	0	15	-1.615e-5	12	NC	1	NC	1
92			min	0	15	-.007	3	-.001	1	-5.231e-4	1	NC	1	NC	1
93		9	max	0	1	.008	2	0	15	-1.615e-5	12	NC	1	NC	1
94			min	0	15	-.007	3	-.001	1	-5.231e-4	1	NC	1	NC	1
95		10	max	0	1	.007	2	0	15	-1.615e-5	12	NC	1	NC	1
96			min	0	15	-.006	3	0	1	-5.231e-4	1	NC	1	NC	1
97		11	max	0	1	.006	2	0	15	-1.615e-5	12	NC	1	NC	1
98			min	0	15	-.005	3	0	1	-5.231e-4	1	NC	1	NC	1
99		12	max	0	1	.005	2	0	15	-1.615e-5	12	NC	1	NC	1
100			min	0	15	-.005	3	0	1	-5.231e-4	1	NC	1	NC	1
101		13	max	0	1	.005	2	0	15	-1.615e-5	12	NC	1	NC	1
102			min	0	15	-.004	3	0	1	-5.231e-4	1	NC	1	NC	1
103		14	max	0	1	.004	2	0	15	-1.615e-5	12	NC	1	NC	1
104			min	0	15	-.003	3	0	1	-5.231e-4	1	NC	1	NC	1
105		15	max	0	1	.003	2	0	15	-1.615e-5	12	NC	1	NC	1
106			min	0	15	-.003	3	0	1	-5.231e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	15	-1.615e-5	12	NC	1	NC	1
108			min	0	15	-.002	3	0	1	-5.231e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.002	2	0	15	-1.615e-5	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-5.231e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.615e-5	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-5.231e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.615e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-5.231e-4	1	NC	1	NC	1
115	M6	1	max	.007	1	.034	2	.003	1	4.412e-4	3	NC	3	NC	1
116			min	-.012	3	-.033	3	-.006	3	-6.38e-7	1	1234.752	2	7657.329	3
117		2	max	.007	1	.032	2	.003	1	4.267e-4	3	NC	3	NC	1
118			min	-.011	3	-.031	3	-.005	3	-4.097e-6	1	1323.041	2	8104.268	3
119		3	max	.006	1	.03	2	.003	1	4.121e-4	3	NC	3	NC	1
120			min	-.011	3	-.03	3	-.005	3	-7.555e-6	1	1424.446	2	8637.841	3
121		4	max	.006	1	.028	2	.002	1	3.976e-4	3	NC	3	NC	1
122			min	-.01	3	-.028	3	-.005	3	-1.101e-5	1	1541.589	2	9275.999	3
123		5	max	.006	1	.025	2	.002	1	3.831e-4	3	NC	3	NC	1
124			min	-.009	3	-.026	3	-.004	3	-1.447e-5	1	1677.853	2	NC	1
125		6	max	.005	1	.023	2	.002	1	3.686e-4	3	NC	3	NC	1
126			min	-.009	3	-.024	3	-.004	3	-1.793e-5	1	1837.664	2	NC	1
127		7	max	.005	1	.021	2	.002	1	3.54e-4	3	NC	3	NC	1
128			min	-.008	3	-.023	3	-.004	3	-2.139e-5	1	2026.926	2	NC	1
129		8	max	.004	1	.019	2	.002	1	3.395e-4	3	NC	3	NC	1
130			min	-.007	3	-.021	3	-.003	3	-2.485e-5	1	2253.692	2	NC	1
131		9	max	.004	1	.017	2	.001	1	3.25e-4	3	NC	3	NC	1
132			min	-.007	3	-.019	3	-.003	3	-2.831e-5	1	2529.231	2	NC	1
133		10	max	.004	1	.015	2	.001	1	3.105e-4	3	NC	3	NC	1
134			min	-.006	3	-.017	3	-.002	3	-3.177e-5	1	2869.81	2	NC	1
135		11	max	.003	1	.013	2	0	1	2.959e-4	3	NC	3	NC	1
136			min	-.005	3	-.015	3	-.002	3	-3.523e-5	1	3299.819	2	NC	1
137		12	max	.003	1	.011	2	0	1	2.814e-4	3	NC	3	NC	1
138			min	-.005	3	-.013	3	-.002	3	-3.868e-5	1	3857.554	2	NC	1
139		13	max	.002	1	.009	2	0	1	2.669e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.001	3	-4.214e-5	1	4606.803	2	NC	1
141		14	max	.002	1	.007	2	0	1	2.524e-4	3	NC	1	NC	1
142			min	-.003	3	-.01	3	-.001	3	-4.56e-5	1	5662.321	2	NC	1
143		15	max	.002	1	.006	2	0	1	2.378e-4	3	NC	1	NC	1
144			min	-.003	3	-.008	3	0	3	-4.906e-5	1	7253.518	2	NC	1
145		16	max	.001	1	.004	2	0	1	2.233e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-5.252e-5	1	9915.516	2	NC	1
147		17	max	0	1	.003	2	0	1	2.088e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-5.598e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.943e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-5.944e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.797e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-6.29e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.989e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-8.562e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	2.567e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.372e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.144e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.181e-5	3	NC	1	NC	1
159		4	max	.001	3	.004	2	.001	3	1.722e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.99e-5	3	NC	1	NC	1
161		5	max	.002	3	.005	2	.001	3	1.3e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	0	10	8545.612	2	NC	1
163		6	max	.002	3	.007	2	.002	3	2.391e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	0	10	6842.696	2	NC	1
165		7	max	.003	3	.008	2	.002	3	4.582e-5	3	NC	1	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.012	3	0	1	0	10	5679.074	2	NC	1
167		8	max	.003	3	.01	2	.002	3	6.773e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-3.381e-7	13	4826.842	2	NC	1
169		9	max	.003	3	.011	2	.002	3	8.964e-5	3	NC	3	NC	1
170			min	-.004	2	-.015	3	-.001	1	-3.901e-6	1	4172.403	2	NC	1
171		10	max	.004	3	.013	2	.003	3	1.115e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-8.125e-6	1	3652.757	2	NC	1
173		11	max	.004	3	.014	2	.003	3	1.335e-4	3	NC	3	NC	1
174			min	-.005	2	-.018	3	-.001	1	-1.235e-5	1	3230.053	2	NC	1
175		12	max	.005	3	.016	2	.003	3	1.554e-4	3	NC	3	NC	1
176			min	-.005	2	-.019	3	-.001	1	-1.657e-5	1	2880.056	2	NC	1
177		13	max	.005	3	.018	2	.003	3	1.773e-4	3	NC	3	NC	1
178			min	-.006	2	-.02	3	-.001	1	-2.08e-5	1	2586.412	2	NC	1
179		14	max	.005	3	.02	2	.003	3	1.992e-4	3	NC	3	NC	1
180			min	-.006	2	-.022	3	-.001	1	-2.502e-5	1	2337.593	2	NC	1
181		15	max	.006	3	.022	2	.003	3	2.211e-4	3	NC	3	NC	1
182			min	-.007	2	-.023	3	-.002	1	-2.925e-5	1	2125.17	2	NC	1
183		16	max	.006	3	.024	2	.003	3	2.43e-4	3	NC	3	NC	1
184			min	-.007	2	-.024	3	-.002	1	-3.347e-5	1	1942.791	2	NC	1
185		17	max	.007	3	.026	2	.003	3	2.649e-4	3	NC	3	NC	1
186			min	-.008	2	-.025	3	-.002	1	-3.769e-5	1	1785.551	2	NC	1
187		18	max	.007	3	.028	2	.003	3	2.868e-4	3	NC	3	NC	1
188			min	-.008	2	-.026	3	-.002	1	-4.192e-5	1	1649.587	2	NC	1
189		19	max	.008	3	.03	2	.003	3	3.087e-4	3	NC	3	NC	1
190			min	-.009	2	-.026	3	-.002	1	-4.614e-5	1	1531.818	2	NC	1
191	M8	1	max	.004	1	.04	2	.002	1	-1.085e-7	10	NC	1	NC	1
192			min	0	15	-.033	3	-.002	3	-2.358e-4	3	NC	1	NC	1
193		2	max	.004	1	.038	2	.002	1	-1.085e-7	10	NC	1	NC	1
194			min	0	15	-.031	3	-.002	3	-2.358e-4	3	NC	1	NC	1
195		3	max	.003	1	.035	2	.001	1	-1.085e-7	10	NC	1	NC	1
196			min	0	15	-.029	3	-.002	3	-2.358e-4	3	NC	1	NC	1
197		4	max	.003	1	.033	2	.001	1	-1.085e-7	10	NC	1	NC	1
198			min	0	15	-.027	3	-.001	3	-2.358e-4	3	NC	1	NC	1
199		5	max	.003	1	.031	2	.001	1	-1.085e-7	10	NC	1	NC	1
200			min	0	15	-.025	3	-.001	3	-2.358e-4	3	NC	1	NC	1
201		6	max	.003	1	.029	2	.001	1	-1.085e-7	10	NC	1	NC	1
202			min	0	15	-.023	3	-.001	3	-2.358e-4	3	NC	1	NC	1
203		7	max	.003	1	.027	2	0	1	-1.085e-7	10	NC	1	NC	1
204			min	0	15	-.022	3	0	3	-2.358e-4	3	NC	1	NC	1
205		8	max	.002	1	.024	2	0	1	-1.085e-7	10	NC	1	NC	1
206			min	0	15	-.02	3	0	3	-2.358e-4	3	NC	1	NC	1
207		9	max	.002	1	.022	2	0	1	-1.085e-7	10	NC	1	NC	1
208			min	0	15	-.018	3	0	3	-2.358e-4	3	NC	1	NC	1
209		10	max	.002	1	.02	2	0	1	-1.085e-7	10	NC	1	NC	1
210			min	0	15	-.016	3	0	3	-2.358e-4	3	NC	1	NC	1
211		11	max	.002	1	.018	2	0	1	-1.085e-7	10	NC	1	NC	1
212			min	0	15	-.014	3	0	3	-2.358e-4	3	NC	1	NC	1
213		12	max	.002	1	.015	2	0	1	-1.085e-7	10	NC	1	NC	1
214			min	0	15	-.013	3	0	3	-2.358e-4	3	NC	1	NC	1
215		13	max	.001	1	.013	2	0	1	-1.085e-7	10	NC	1	NC	1
216			min	0	15	-.011	3	0	3	-2.358e-4	3	NC	1	NC	1
217		14	max	.001	1	.011	2	0	1	-1.085e-7	10	NC	1	NC	1
218			min	0	15	-.009	3	0	3	-2.358e-4	3	NC	1	NC	1
219		15	max	0	1	.009	2	0	1	-1.085e-7	10	NC	1	NC	1
220			min	0	15	-.007	3	0	3	-2.358e-4	3	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-1.085e-7	10	NC	1	NC	1
222			min	0	15	-.005	3	0	3	-2.358e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	1	-1.085e-7	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.358e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.085e-7	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.358e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.085e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.358e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.012	2	0	3	6.484e-4	1	NC	3	NC	1
230			min	-.003	3	-.012	3	-.002	1	-4.865e-4	3	3657.447	2	NC	1
231		2	max	.002	1	.011	2	0	3	6.147e-4	1	NC	3	NC	1
232			min	-.003	3	-.011	3	-.002	1	-4.691e-4	3	3990.681	2	NC	1
233		3	max	.002	1	.01	2	0	3	5.81e-4	1	NC	3	NC	1
234			min	-.003	3	-.011	3	-.001	1	-4.518e-4	3	4386.636	2	NC	1
235		4	max	.002	1	.009	2	0	3	5.473e-4	1	NC	3	NC	1
236			min	-.003	3	-.01	3	-.001	1	-4.344e-4	3	4860.16	2	NC	1
237		5	max	.002	1	.008	2	0	3	5.136e-4	1	NC	1	NC	1
238			min	-.003	3	-.01	3	-.001	1	-4.171e-4	3	5430.947	2	NC	1
239		6	max	.002	1	.007	2	0	3	4.8e-4	1	NC	1	NC	1
240			min	-.002	3	-.009	3	-.001	1	-3.997e-4	3	6125.52	2	NC	1
241		7	max	.002	1	.006	2	0	3	4.463e-4	1	NC	1	NC	1
242			min	-.002	3	-.009	3	-.001	1	-3.823e-4	3	6980.256	2	NC	1
243		8	max	.001	1	.005	2	0	3	4.126e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	-.001	1	-3.65e-4	3	8046.116	2	NC	1
245		9	max	.001	1	.005	2	0	3	3.789e-4	1	NC	1	NC	1
246			min	-.002	3	-.008	3	0	1	-3.476e-4	3	9396.301	2	NC	1
247		10	max	.001	1	.004	2	0	3	3.452e-4	1	NC	1	NC	1
248			min	-.002	3	-.007	3	0	1	-3.303e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	3.116e-4	1	NC	1	NC	1
250			min	-.001	3	-.006	3	0	1	-3.129e-4	3	NC	1	NC	1
251		12	max	0	1	.003	2	0	3	2.779e-4	1	NC	1	NC	1
252			min	-.001	3	-.006	3	0	1	-2.956e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	2.442e-4	1	NC	1	NC	1
254			min	-.001	3	-.005	3	0	1	-2.782e-4	3	NC	1	NC	1
255		14	max	0	1	.002	2	0	3	2.105e-4	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-2.609e-4	3	NC	1	NC	1
257		15	max	0	1	.001	2	0	3	1.769e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.435e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.432e-4	1	NC	1	NC	1
260			min	0	3	-.003	3	0	1	-2.262e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.095e-4	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-2.088e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	7.581e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.914e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	4.214e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.741e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.332e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.066e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	2	6.124e-5	3	NC	1	NC	1
270			min	0	2	-.001	3	0	3	-4.344e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	3.915e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.621e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	1.706e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-8.899e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	-3.568e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-1.118e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-6.699e-6	15	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-1.345e-4	1	NC	1	NC	1
279		7	max	0	3	.001	2	0	10	-7.816e-6	15	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.002	2	0	1	4.079e-4	1	NC	1	NC	1
338			min	0	15	-.001	3	0	15	1.992e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.079e-4	1	NC	1	NC	1
340			min	0	15	0	3	0	15	1.992e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.079e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.992e-5	15	NC	1	NC	1
343	M1	1	max	.01	3	.029	3	.003	3	5.577e-3	2	NC	1	NC	1
344			min	-.01	2	-.025	2	-.003	1	-7.978e-3	3	NC	1	NC	1
345		2	max	.01	3	.018	3	.002	3	2.704e-3	2	NC	4	NC	1
346			min	-.01	2	-.015	2	-.006	1	-3.945e-3	3	4532.425	2	NC	1
347		3	max	.01	3	.007	3	.002	3	1.409e-5	3	NC	4	NC	2
348			min	-.01	2	-.005	2	-.008	1	-4.067e-4	1	2332.835	2	9277.164	1
349		4	max	.01	3	.003	2	.001	3	1.771e-5	3	NC	4	NC	2
350			min	-.01	2	-.002	3	-.009	1	-3.522e-4	1	1632.434	2	7695.966	1
351		5	max	.01	3	.011	2	.001	3	2.134e-5	3	NC	4	NC	2
352			min	-.01	2	-.01	3	-.009	1	-2.977e-4	1	1293.81	2	7415.737	1
353		6	max	.01	3	.017	2	0	3	2.497e-5	3	NC	4	NC	2
354			min	-.01	2	-.016	3	-.009	1	-2.432e-4	1	1100.977	2	7976.117	1
355		7	max	.01	3	.022	2	0	3	2.859e-5	3	NC	5	NC	2
356			min	-.01	2	-.021	3	-.008	1	-1.887e-4	1	982.906	2	9578.098	1
357		8	max	.01	3	.026	2	0	3	3.222e-5	3	NC	5	NC	1
358			min	-.01	2	-.024	3	-.006	1	-1.342e-4	1	910.015	2	NC	1
359		9	max	.01	3	.029	2	0	3	3.585e-5	3	NC	5	NC	1
360			min	-.01	2	-.026	3	-.004	1	-7.967e-5	1	868.51	2	NC	1
361		10	max	.01	3	.03	2	0	3	3.948e-5	3	NC	5	NC	1
362			min	-.01	2	-.027	3	-.003	1	-2.516e-5	1	852.105	2	NC	1
363		11	max	.01	3	.029	2	0	3	4.31e-5	3	NC	5	NC	1
364			min	-.01	2	-.026	3	0	1	1.232e-6	15	859.104	2	NC	1
365		12	max	.01	3	.027	2	.001	1	8.386e-5	1	NC	5	NC	1
366			min	-.01	2	-.024	3	0	15	3.812e-6	15	891.802	2	NC	1
367		13	max	.01	3	.024	2	.003	1	1.384e-4	1	NC	5	NC	2
368			min	-.01	2	-.02	3	0	15	6.392e-6	15	957.734	2	9774.936	1
369		14	max	.01	3	.018	2	.004	1	1.929e-4	1	NC	4	NC	2
370			min	-.01	2	-.015	3	0	15	8.972e-6	15	1074.101	2	8098.248	1
371		15	max	.01	3	.011	2	.004	1	2.474e-4	1	NC	4	NC	2
372			min	-.01	2	-.009	3	0	15	1.155e-5	15	1281.153	2	7505.839	1
373		16	max	.01	3	.002	2	.004	1	2.84e-4	1	NC	4	NC	2
374			min	-.01	2	-.002	3	0	15	1.329e-5	15	1691.286	2	7769.378	1
375		17	max	.01	3	.006	3	.003	1	5.475e-5	3	NC	4	NC	2
376			min	-.01	2	-.008	2	0	15	-1.063e-4	1	2453.569	3	9351.227	1
377		18	max	.01	3	.016	3	.001	1	4.1e-3	2	NC	2	NC	1
378			min	-.01	2	-.021	2	0	15	-2.09e-3	3	4794.256	3	NC	1
379		19	max	.01	3	.026	3	0	3	8.28e-3	2	NC	1	NC	1
380			min	-.01	2	-.034	2	-.002	1	-4.275e-3	3	5256.823	2	NC	1
381	M5	1	max	.028	3	.081	3	.003	3	4.53e-6	3	NC	1	NC	1
382			min	-.031	2	-.07	2	-.003	1	0	2	3539.221	3	NC	1
383		2	max	.028	3	.05	3	.004	3	1.265e-4	3	NC	4	NC	1
384			min	-.031	2	-.043	2	-.003	1	-5.089e-5	1	1690.11	2	NC	1
385		3	max	.028	3	.02	3	.006	3	2.46e-4	3	NC	5	NC	1
386			min	-.031	2	-.016	2	-.003	1	-1.009e-4	1	864.59	2	NC	1
387		4	max	.028	3	.007	2	.006	3	2.359e-4	3	NC	5	NC	1
388			min	-.031	2	-.004	3	-.003	1	-9.634e-5	1	603.166	2	NC	1
389		5	max	.028	3	.028	2	.007	3	2.258e-4	3	NC	5	NC	1
390			min	-.031	2	-.025	3	-.003	1	-9.175e-5	1	476.805	2	NC	1
391		6	max	.028	3	.045	2	.007	3	2.157e-4	3	NC	5	NC	1
392			min	-.031	2	-.042	3	-.003	1	-8.715e-5	1	404.794	2	9542.642	3
393		7	max	.028	3	.06	2	.007	3	2.056e-4	3	NC	5	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.031	2	-.054	3	-.003	1	-8.255e-5	1	360.618	2	9059.483	3
395		8	max	.028	3	.07	2	.007	3	1.955e-4	3	NC	5	NC	1
396			min	-.031	2	-.063	3	-.003	1	-7.796e-5	1	333.234	2	8945.378	3
397		9	max	.028	3	.077	2	.007	3	1.854e-4	3	NC	5	NC	1
398			min	-.031	2	-.069	3	-.003	1	-7.336e-5	1	317.487	2	9129.169	3
399		10	max	.028	3	.08	2	.007	3	1.753e-4	3	NC	5	NC	1
400			min	-.031	2	-.07	3	-.002	1	-6.876e-5	1	311.02	2	9601.345	3
401		11	max	.027	3	.079	2	.006	3	1.652e-4	3	NC	5	NC	1
402			min	-.031	2	-.068	3	-.002	1	-6.417e-5	1	313.178	2	NC	1
403		12	max	.027	3	.074	2	.006	3	1.551e-4	3	NC	5	NC	1
404			min	-.031	2	-.062	3	-.002	1	-5.957e-5	1	324.788	2	NC	1
405		13	max	.027	3	.064	2	.005	3	1.45e-4	3	NC	5	NC	1
406			min	-.031	2	-.053	3	-.002	1	-5.497e-5	1	348.614	2	NC	1
407		14	max	.027	3	.049	2	.004	3	1.349e-4	3	NC	5	NC	1
408			min	-.031	2	-.04	3	-.002	1	-5.038e-5	1	391.019	2	NC	1
409		15	max	.027	3	.03	2	.003	3	1.248e-4	3	NC	5	NC	1
410			min	-.031	2	-.024	3	-.002	1	-4.578e-5	1	466.996	2	NC	1
411		16	max	.027	3	.005	2	.003	3	1.101e-4	3	NC	5	NC	1
412			min	-.031	2	-.005	3	-.002	1	-4.531e-5	1	618.923	2	NC	1
413		17	max	.027	3	.018	3	.002	3	-7.542e-8	10	NC	5	NC	1
414			min	-.031	2	-.025	2	-.002	1	-1.431e-4	1	934.635	3	NC	1
415		18	max	.027	3	.042	3	.001	3	-9.038e-8	10	NC	4	NC	1
416			min	-.031	2	-.059	2	-.002	1	-7.314e-5	1	1838.096	3	NC	1
417		19	max	.027	3	.068	3	0	3	-3.334e-8	15	NC	3	NC	1
418			min	-.031	2	-.096	2	-.002	1	-8.123e-7	3	1829.376	2	NC	1
419	M9	1	max	.01	3	.029	3	.003	3	7.988e-3	3	NC	1	NC	1
420			min	-.01	2	-.025	2	-.004	1	-5.577e-3	2	NC	1	NC	1
421		2	max	.01	3	.017	3	.001	3	3.915e-3	3	NC	4	NC	1
422			min	-.01	2	-.015	2	0	9	-2.718e-3	2	4534.784	2	NC	1
423		3	max	.01	3	.006	3	.002	1	1.886e-4	1	NC	4	NC	1
424			min	-.01	2	-.005	2	0	3	-8.219e-5	3	2334.089	2	NC	1
425		4	max	.01	3	.003	2	.003	1	1.427e-4	1	NC	4	NC	1
426			min	-.01	2	-.003	3	-.001	3	-8.449e-5	3	1633.326	2	NC	1
427		5	max	.01	3	.011	2	.003	1	9.683e-5	1	NC	4	NC	1
428			min	-.01	2	-.01	3	-.002	3	-8.679e-5	3	1294.509	2	NC	1
429		6	max	.01	3	.017	2	.003	1	5.094e-5	1	NC	4	NC	1
430			min	-.01	2	-.016	3	-.003	3	-8.909e-5	3	1101.557	2	9263.123	3
431		7	max	.01	3	.022	2	.002	1	2.02e-5	2	NC	5	NC	1
432			min	-.01	2	-.021	3	-.004	3	-9.139e-5	3	983.406	2	8396.759	3
433		8	max	.01	3	.026	2	0	2	3.699e-6	10	NC	5	NC	1
434			min	-.01	2	-.024	3	-.004	3	-9.369e-5	3	910.459	2	7893.992	3
435		9	max	.01	3	.029	2	0	2	-5.079e-7	10	NC	5	NC	1
436			min	-.01	2	-.026	3	-.004	3	-9.599e-5	3	868.913	2	7645.385	3
437		10	max	.01	3	.03	2	0	10	-4.715e-6	10	NC	5	NC	1
438			min	-.01	2	-.027	3	-.005	3	-1.326e-4	1	852.477	2	7602.305	3
439		11	max	.01	3	.029	2	0	10	-8.923e-6	10	NC	5	NC	1
440			min	-.01	2	-.026	3	-.005	3	-1.785e-4	1	859.451	2	7752.541	3
441		12	max	.01	3	.027	2	0	15	-1.111e-5	15	NC	5	NC	1
442			min	-.01	2	-.024	3	-.006	1	-2.244e-4	1	892.127	2	8114.357	3
443		13	max	.01	3	.024	2	0	15	-1.33e-5	15	NC	5	NC	2
444			min	-.01	2	-.02	3	-.007	1	-2.702e-4	1	958.036	2	8559.5	1
445		14	max	.01	3	.018	2	0	15	-1.549e-5	15	NC	4	NC	2
446			min	-.01	2	-.015	3	-.008	1	-3.161e-4	1	1074.366	2	7418.304	1
447		15	max	.01	3	.011	2	0	15	-1.767e-5	15	NC	4	NC	2
448			min	-.01	2	-.009	3	-.008	1	-3.62e-4	1	1281.339	2	7062.829	1
449		16	max	.01	3	.002	2	0	15	-1.933e-5	15	NC	4	NC	2
450			min	-.01	2	-.002	3	-.008	1	-3.973e-4	1	1687.483	3	7439.406	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.01	3	.006	3	0	15	1.011e-4	3	NC	4	NC	2
452			min	-.01	2	-.008	2	-.007	1	-1.795e-4	1	2443.348	3	9061.972	1
453		18	max	.01	3	.016	3	0	15	2.17e-3	3	NC	2	NC	1
454			min	-.01	2	-.021	2	-.004	1	-4.114e-3	2	4774.816	3	NC	1
455		19	max	.01	3	.026	3	0	3	4.273e-3	3	NC	1	NC	1
456			min	-.01	2	-.034	2	-.001	1	-8.28e-3	2	5270.056	2	NC	1
457	M13	1	max	.004	1	.029	3	.01	3	4.412e-3	3	NC	1	NC	1
458			min	-.003	3	-.025	2	-.01	2	-3.954e-3	2	NC	1	NC	1
459		2	max	.004	1	.085	3	.01	9	5.232e-3	3	NC	4	NC	2
460			min	-.003	3	-.065	2	-.005	10	-4.679e-3	2	2332.428	3	8471.879	1
461		3	max	.004	1	.133	3	.029	1	6.053e-3	3	NC	4	NC	2
462			min	-.003	3	-.098	2	-.004	10	-5.404e-3	2	1270.935	3	3799.317	1
463		4	max	.004	1	.164	3	.044	1	6.874e-3	3	NC	5	NC	3
464			min	-.003	3	-.121	2	-.004	10	-6.128e-3	2	974.428	3	2652.387	1
465		5	max	.003	1	.177	3	.051	1	7.694e-3	3	NC	5	NC	3
466			min	-.003	3	-.131	2	-.004	10	-6.853e-3	2	889.921	3	2363.976	1
467		6	max	.003	1	.172	3	.046	1	8.515e-3	3	NC	5	NC	2
468			min	-.003	3	-.128	2	-.007	10	-7.578e-3	2	925.428	3	2585.604	1
469		7	max	.003	1	.151	3	.031	1	9.335e-3	3	NC	5	NC	2
470			min	-.003	3	-.116	2	-.01	10	-8.303e-3	2	1083.416	3	3625.524	1
471		8	max	.003	1	.122	3	.022	3	1.016e-2	3	NC	4	NC	2
472			min	-.003	3	-.097	2	-.017	2	-9.028e-3	2	1424.938	3	8346.171	1
473		9	max	.003	1	.094	3	.025	3	1.098e-2	3	NC	4	NC	1
474			min	-.003	3	-.079	2	-.027	2	-9.753e-3	2	2030.72	3	8199.451	2
475		10	max	.003	1	.081	3	.028	3	1.18e-2	3	NC	4	NC	1
476			min	-.003	3	-.07	2	-.031	2	-1.048e-2	2	2530.455	3	6473.521	2
477		11	max	.003	1	.094	3	.031	3	1.098e-2	3	NC	4	NC	1
478			min	-.003	3	-.079	2	-.027	2	-9.753e-3	2	2030.718	3	6385.685	3
479		12	max	.003	1	.122	3	.032	3	1.016e-2	3	NC	4	NC	2
480			min	-.003	3	-.097	2	-.017	2	-9.029e-3	2	1424.937	3	5978.394	3
481		13	max	.003	1	.151	3	.032	3	9.34e-3	3	NC	5	NC	2
482			min	-.003	3	-.116	2	-.01	10	-8.304e-3	2	1083.415	3	3609.027	1
483		14	max	.003	1	.172	3	.046	1	8.522e-3	3	NC	5	NC	2
484			min	-.003	3	-.128	2	-.007	10	-7.579e-3	2	925.427	3	2583.39	1
485		15	max	.003	1	.177	3	.05	1	7.703e-3	3	NC	5	NC	5
486			min	-.003	3	-.131	2	-.004	10	-6.854e-3	2	889.92	3	2368.108	1
487		16	max	.003	1	.165	3	.044	1	6.884e-3	3	NC	5	NC	3
488			min	-.003	3	-.121	2	-.004	10	-6.13e-3	2	974.427	3	2663.883	1
489		17	max	.003	1	.133	3	.029	1	6.065e-3	3	NC	4	NC	2
490			min	-.003	3	-.098	2	-.004	10	-5.405e-3	2	1270.934	3	3828.614	1
491		18	max	.003	1	.086	3	.014	3	5.246e-3	3	NC	4	NC	2
492			min	-.003	3	-.065	2	-.005	10	-4.68e-3	2	2332.426	3	8586.727	1
493		19	max	.003	1	.029	3	.01	3	4.428e-3	3	NC	1	NC	1
494			min	-.003	3	-.025	2	-.01	2	-3.956e-3	2	NC	1	NC	1
495	M16	1	max	.001	1	.026	3	.01	3	5.09e-3	2	NC	1	NC	1
496			min	0	3	-.034	2	-.01	2	-3.762e-3	3	NC	1	NC	1
497		2	max	.001	1	.056	3	.014	3	6.055e-3	2	NC	4	NC	2
498			min	0	3	-.093	2	-.005	10	-4.418e-3	3	2247.088	2	8474.18	1
499		3	max	.001	1	.082	3	.029	1	7.02e-3	2	NC	4	NC	2
500			min	0	3	-.142	2	-.004	10	-5.073e-3	3	1222.091	2	3800.018	1
501		4	max	.001	1	.1	3	.044	1	7.985e-3	2	NC	5	NC	3
502			min	0	3	-.176	2	-.003	10	-5.728e-3	3	933.804	2	2652.813	1
503		5	max	.001	1	.108	3	.05	1	8.95e-3	2	NC	5	NC	5
504			min	0	3	-.19	2	-.004	10	-6.383e-3	3	848.163	2	2364.417	1
505		6	max	.002	1	.108	3	.046	1	9.915e-3	2	NC	5	NC	2
506			min	0	3	-.185	2	-.007	10	-7.039e-3	3	874.279	2	2586.371	1
507		7	max	.002	1	.099	3	.031	1	1.088e-2	2	NC	5	NC	2



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.165	2	-.01	10	-7.694e-3	3	1008.699	2	3627.874	1
509	8	max	.002	1	.087	3	.029	3	1.184e-2	2	NC	4	NC	2
510		min	0	3	-.136	2	-.017	2	-8.349e-3	3	1293.851	2	6726.222	3
511	9	max	.002	1	.074	3	.028	3	1.281e-2	2	NC	4	NC	1
512		min	0	3	-.109	2	-.026	2	-9.004e-3	3	1771.559	2	7090.714	3
513	10	max	.002	1	.068	3	.027	3	1.377e-2	2	NC	4	NC	1
514		min	0	3	-.096	2	-.031	2	-9.66e-3	3	2140.035	2	6534.006	2
515	11	max	.002	1	.074	3	.026	3	1.281e-2	2	NC	4	NC	1
516		min	0	3	-.109	2	-.026	2	-9.003e-3	3	1771.559	2	8298.638	2
517	12	max	.002	1	.086	3	.025	3	1.185e-2	2	NC	4	NC	2
518		min	0	3	-.136	2	-.017	2	-8.346e-3	3	1293.851	2	8297.402	1
519	13	max	.002	1	.099	3	.031	1	1.088e-2	2	NC	5	NC	2
520		min	0	3	-.165	2	-.01	10	-7.689e-3	3	1008.699	2	3623.595	1
521	14	max	.002	1	.108	3	.045	1	9.916e-3	2	NC	5	NC	2
522		min	0	3	-.185	2	-.007	10	-7.032e-3	3	874.279	2	2590.963	1
523	15	max	.002	1	.108	3	.05	1	8.951e-3	2	NC	5	NC	3
524		min	0	3	-.19	2	-.004	10	-6.375e-3	3	848.163	2	2374.142	1
525	16	max	.002	1	.1	3	.044	1	7.986e-3	2	NC	5	NC	3
526		min	0	3	-.176	2	-.003	10	-5.718e-3	3	933.804	2	2670.536	1
527	17	max	.002	1	.082	3	.029	1	7.022e-3	2	NC	4	NC	2
528		min	0	3	-.142	2	-.004	10	-5.061e-3	3	1222.091	2	3839.096	1
529	18	max	.002	1	.056	3	.012	3	6.057e-3	2	NC	4	NC	2
530		min	0	3	-.093	2	-.005	10	-4.404e-3	3	2247.088	2	8616.529	1
531	19	max	.002	1	.026	3	.01	3	5.092e-3	2	NC	1	NC	1
532		min	0	3	-.034	2	-.01	2	-3.747e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	4.247e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.87e-5	2	NC	1	NC	1
535	2	max	0	3	-.002	15	0	1	8.706e-4	3	NC	1	NC	1
536		min	0	2	-.008	4	0	3	-5.203e-4	2	NC	1	NC	1
537	3	max	0	3	-.004	15	.003	1	1.317e-3	3	NC	5	NC	1
538		min	0	2	-.015	4	-.003	3	-9.619e-4	2	5246.74	4	NC	1
539	4	max	0	3	-.005	15	.006	1	1.763e-3	3	NC	15	NC	4
540		min	0	2	-.022	4	-.007	3	-1.403e-3	2	3599.57	4	5999.32	3
541	5	max	0	3	-.007	15	.01	2	2.209e-3	3	NC	15	NC	4
542		min	0	2	-.028	4	-.012	3	-1.845e-3	2	2808.781	4	3955.867	3
543	6	max	0	3	-.008	15	.014	2	2.655e-3	3	NC	15	NC	4
544		min	-.001	2	-.033	4	-.017	3	-2.287e-3	2	2363.886	4	2889.641	3
545	7	max	0	3	-.009	15	.019	2	3.1e-3	3	8918.133	15	NC	4
546		min	-.001	2	-.038	4	-.023	3	-2.728e-3	2	2096.34	4	2264.237	3
547	8	max	0	3	-.01	15	.023	2	3.546e-3	3	8235.056	15	NC	4
548		min	-.001	2	-.041	4	-.028	3	-3.17e-3	2	1935.773	4	1870.251	3
549	9	max	0	3	-.01	15	.027	2	3.992e-3	3	7867.383	15	NC	4
550		min	-.002	2	-.043	4	-.033	3	-3.611e-3	2	1849.346	4	1612.079	3
551	10	max	0	3	-.01	15	.03	2	4.438e-3	3	7751.069	15	NC	4
552		min	-.002	2	-.044	4	-.037	3	-4.053e-3	2	1822.005	4	1441.591	3
553	11	max	.001	3	-.01	15	.032	2	4.884e-3	3	7867.383	15	NC	4
554		min	-.002	2	-.043	4	-.039	3	-4.494e-3	2	1849.346	4	1333.49	3
555	12	max	.001	3	-.01	15	.032	2	5.33e-3	3	8235.056	15	NC	4
556		min	-.002	2	-.041	4	-.04	3	-4.936e-3	2	1935.773	4	1275.254	3
557	13	max	.001	3	-.009	15	.03	2	5.776e-3	3	8918.133	15	NC	4
558		min	-.002	2	-.038	4	-.038	3	-5.378e-3	2	2096.34	4	1263.335	3
559	14	max	.001	3	-.008	15	.028	1	6.222e-3	3	NC	15	NC	4
560		min	-.003	2	-.034	4	-.035	3	-5.819e-3	2	2363.886	4	1303.362	3
561	15	max	.001	3	-.007	15	.023	1	6.668e-3	3	NC	15	NC	4
562		min	-.003	2	-.029	4	-.028	3	-6.261e-3	2	2808.781	4	1415.687	3
563	16	max	.002	3	-.005	15	.016	1	7.114e-3	3	NC	15	NC	4
564		min	-.003	2	-.023	4	-.018	3	-6.702e-3	2	3599.57	4	1655.443	3



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.002	3	-.004	15	.006	1	7.56e-3	3	NC	5	NC	4
566		min	-.003	2	-.016	4	-.004	3	-7.144e-3	2	5246.74	4	2195.494	3
567	18	max	.002	3	0	2	.014	3	8.006e-3	3	NC	1	NC	4
568		min	-.003	2	-.009	4	-.015	2	-7.585e-3	2	NC	1	3910.186	3
569	19	max	.002	3	.007	2	.036	3	8.452e-3	3	NC	1	NC	1
570		min	-.004	2	-.002	9	-.034	2	-8.027e-3	2	NC	1	NC	1
571	M16A	1	max	0	.002	2	.012	3	2.923e-3	3	NC	1	NC	1
572		min	-.002	3	-.002	3	-.012	2	-2.828e-3	2	NC	1	NC	1
573	2	max	0	10	-.002	15	.003	3	2.804e-3	3	NC	1	NC	1
574		min	-.002	3	-.008	4	-.004	2	-2.698e-3	2	NC	1	9582.077	3
575	3	max	0	10	-.004	15	.007	1	2.685e-3	3	NC	5	NC	4
576		min	-.002	3	-.015	4	-.004	3	-2.568e-3	2	5246.74	4	5424.713	3
577	4	max	0	10	-.005	15	.011	1	2.566e-3	3	NC	15	NC	4
578		min	-.002	3	-.022	4	-.009	3	-2.438e-3	2	3599.57	4	4128.657	3
579	5	max	0	10	-.007	15	.014	1	2.447e-3	3	NC	15	NC	4
580		min	-.001	3	-.028	4	-.013	3	-2.307e-3	2	2808.781	4	3568.435	3
581	6	max	0	10	-.008	15	.016	1	2.328e-3	3	NC	15	NC	4
582		min	-.001	3	-.034	4	-.015	3	-2.177e-3	2	2363.886	4	3325.716	3
583	7	max	0	10	-.009	15	.017	1	2.209e-3	3	8918.133	15	NC	4
584		min	-.001	3	-.038	4	-.016	3	-2.047e-3	2	2096.34	4	3269.73	3
585	8	max	0	10	-.01	15	.017	1	2.09e-3	3	8235.056	15	NC	4
586		min	-.001	3	-.041	4	-.016	3	-1.917e-3	2	1935.773	4	3356.27	3
587	9	max	0	10	-.01	15	.016	1	1.972e-3	3	7867.383	15	NC	4
588		min	-.001	3	-.043	4	-.015	3	-1.786e-3	2	1849.346	4	3580.391	3
589	10	max	0	10	-.01	15	.015	1	1.853e-3	3	7751.069	15	NC	4
590		min	0	3	-.043	4	-.014	3	-1.656e-3	2	1822.005	4	3965.847	3
591	11	max	0	10	-.01	15	.013	1	1.734e-3	3	7867.383	15	NC	4
592		min	0	3	-.043	4	-.012	3	-1.526e-3	2	1849.346	4	4570.784	3
593	12	max	0	10	-.01	15	.011	1	1.615e-3	3	8235.056	15	NC	4
594		min	0	3	-.041	4	-.01	3	-1.395e-3	2	1935.773	4	5511.095	3
595	13	max	0	10	-.009	15	.008	1	1.496e-3	3	8918.133	15	NC	4
596		min	0	3	-.038	4	-.007	3	-1.265e-3	2	2096.34	4	7020.682	3
597	14	max	0	10	-.008	15	.006	1	1.377e-3	3	NC	15	NC	2
598		min	0	3	-.033	4	-.005	3	-1.135e-3	2	2363.886	4	9615.127	3
599	15	max	0	10	-.007	15	.004	1	1.258e-3	3	NC	15	NC	1
600		min	0	3	-.028	4	-.003	3	-1.005e-3	2	2808.781	4	NC	1
601	16	max	0	10	-.005	15	.002	1	1.139e-3	3	NC	15	NC	1
602		min	0	3	-.022	4	0	3	-8.743e-4	2	3599.57	4	NC	1
603	17	max	0	10	-.004	15	.001	9	1.02e-3	3	NC	5	NC	1
604		min	0	3	-.015	4	0	2	-7.44e-4	2	5246.74	4	NC	1
605	18	max	0	10	-.002	15	0	3	9.014e-4	3	NC	1	NC	1
606		min	0	3	-.008	4	0	2	-6.138e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	7.825e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.835e-4	2	NC	1	NC	1



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Software
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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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