

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

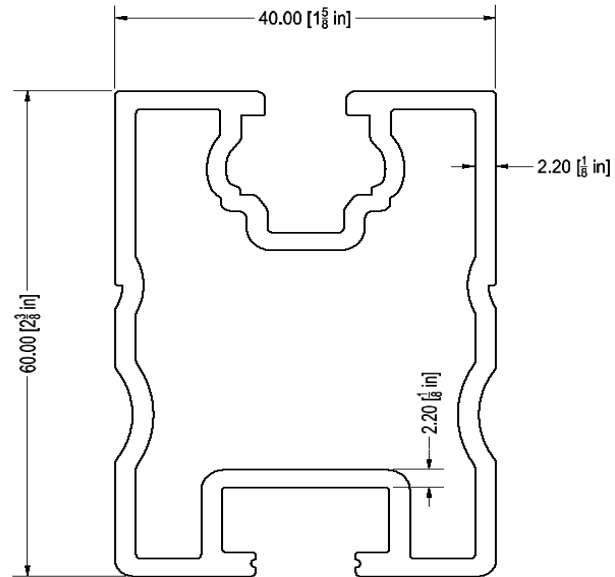
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

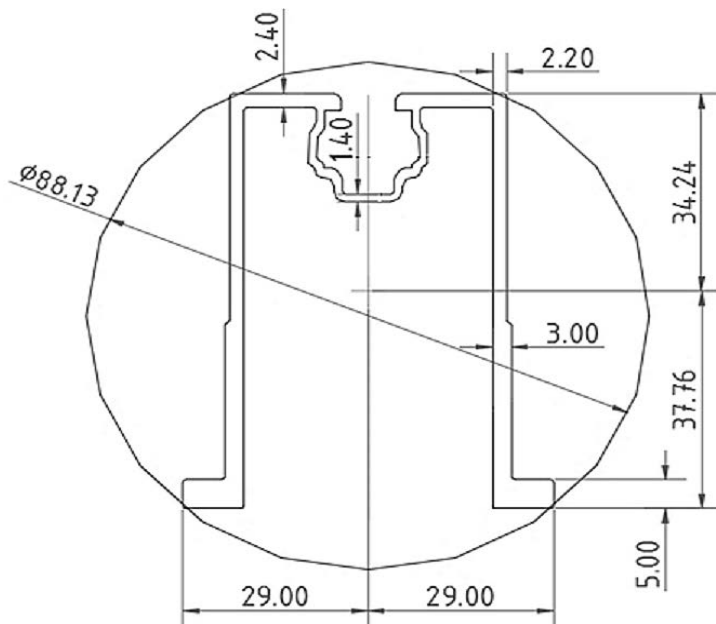
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42 in
ΦF_{ty} STRONG-AXIS =	29.99 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	-0.343 k-ft
M_z =	-0.018 k-ft
$M_{y \text{ allowable}}$ =	1.276 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	29%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

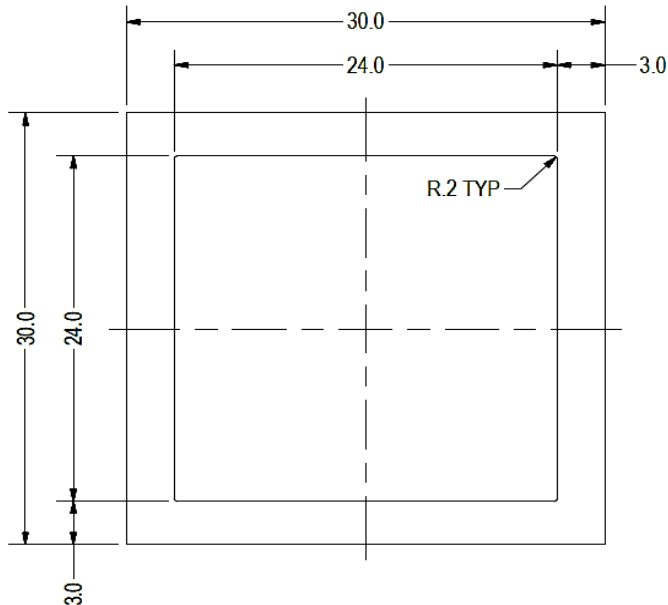
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.76 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.464 k-ft
M_z =	-0.024 k-ft
P_n =	0.242 k
$M_{y \text{ allowable}}$ =	1.460 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.023 k-ft
P_n =	0.124 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.587 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	15%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.608 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.165 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

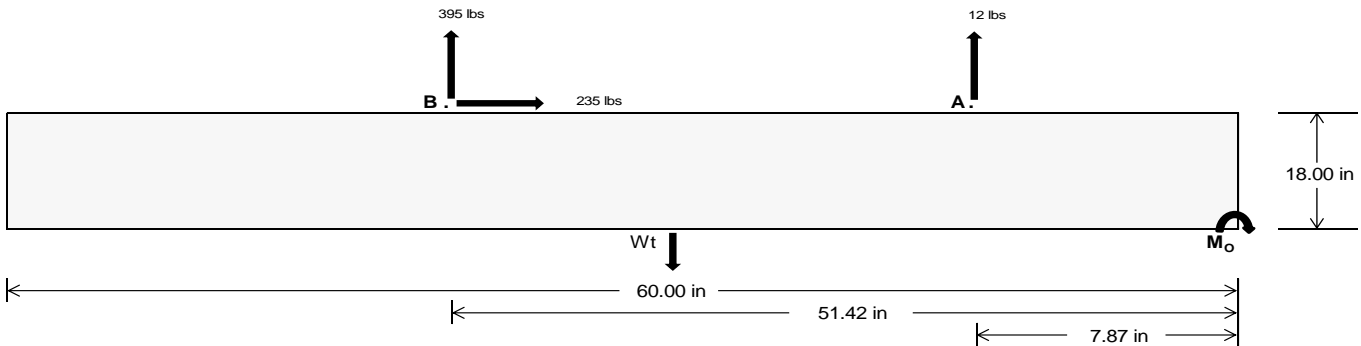
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>58.42</u>	<u>1713.73</u>	k
Compressive Load =	<u>911.11</u>	<u>1100.84</u>	k
Lateral Load =	<u>18.98</u>	<u>1019.73</u>	k
Moment (Weak Axis) =	<u>0.03</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24634.9$ in-lbs
Resisting Force Required = 821.16 lbs
S.F. = 1.67
Weight Required = 1368.61 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 235.26 lbs
Friction = 0.4
Weight Required = 588.16 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 235.26 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

		Ballast Width			
		20 in	21 in	22 in	23 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$		1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	284 lbs	284 lbs	284 lbs	284 lbs	366 lbs	366 lbs	366 lbs	366 lbs	461 lbs	461 lbs	461 lbs	461 lbs	-25 lbs	-25 lbs	-25 lbs	-25 lbs
F_B	188 lbs	188 lbs	188 lbs	188 lbs	473 lbs	473 lbs	473 lbs	473 lbs	477 lbs	477 lbs	477 lbs	477 lbs	-790 lbs	-790 lbs	-790 lbs	-790 lbs
F_V	22 lbs	22 lbs	22 lbs	22 lbs	419 lbs	419 lbs	419 lbs	419 lbs	329 lbs	329 lbs	329 lbs	329 lbs	-471 lbs	-471 lbs	-471 lbs	-471 lbs
P_{total}	2284 lbs	2375 lbs	2465 lbs	2556 lbs	2652 lbs	2742 lbs	2833 lbs	2924 lbs	2751 lbs	2841 lbs	2932 lbs	3023 lbs	273 lbs	327 lbs	382 lbs	436 lbs
M	221 lbs-ft	221 lbs-ft	221 lbs-ft	221 lbs-ft	460 lbs-ft	460 lbs-ft	460 lbs-ft	460 lbs-ft	492 lbs-ft	492 lbs-ft	492 lbs-ft	492 lbs-ft	658 lbs-ft	658 lbs-ft	658 lbs-ft	658 lbs-ft
e	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.41 ft	2.01 ft	1.72 ft	1.51 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	242.3 psf	241.1 psf	240.0 psf	239.0 psf	251.9 psf	250.3 psf	248.8 psf	247.4 psf	259.2 psf	257.2 psf	255.4 psf	253.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	305.9 psf	301.7 psf	297.9 psf	294.4 psf	384.5 psf	376.6 psf	369.3 psf	362.7 psf	401.0 psf	392.3 psf	384.3 psf	377.1 psf	1215.8 psf	254.4 psf	178.8 psf	153.0 psf

Maximum Bearing Pressure = 1216 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

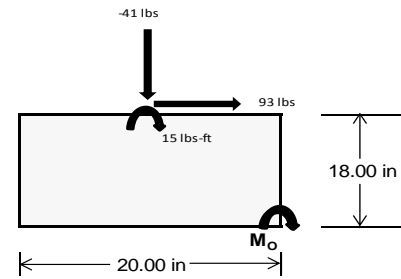
Overturning Check

$M_o = 188.3 \text{ ft-lbs}$
 Resisting Force Required = 225.99 lbs
 S.F. = 1.67
 Weight Required = 376.64 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	115 lbs	38 lbs	51 lbs	194 lbs	337 lbs	146 lbs	81 lbs	-41 lbs	20 lbs
F_v	11 lbs	93 lbs	11 lbs	8 lbs	70 lbs	9 lbs	11 lbs	93 lbs	11 lbs
P_{total}	2359 lbs	2282 lbs	2295 lbs	2330 lbs	2473 lbs	2282 lbs	737 lbs	615 lbs	676 lbs
M	31 lbs-ft	154 lbs-ft	32 lbs-ft	22 lbs-ft	117 lbs-ft	25 lbs-ft	31 lbs-ft	154 lbs-ft	32 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.25 ft	0.05 ft
$L/6$	0.28 ft	1.53 ft	1.64 ft	1.65 ft	1.57 ft	1.64 ft	1.58 ft	1.17 ft	1.57 ft
f_{min}	269.6 sqft	207.1 sqft	261.7 sqft	270.0 sqft	246.4 sqft	263.1 sqft	75.0 sqft	7.2 sqft	67.4 sqft
f_{max}	296.4 psf	340.5 psf	289.2 psf	289.3 psf	347.2 psf	284.6 psf	101.9 psf	140.5 psf	94.8 psf



Maximum Bearing Pressure = 347 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.873 k
Allowable Uplift =	1.214 k
Utilization =	<u>72%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.095 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.701 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>

Diagonal Strut

Maximum Axial Load =	0.587 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>



Rear Strut

Maximum Axial Load =	1.068 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.165 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.047 in
	<u>0.047 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.0 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.276 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &21.4323 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

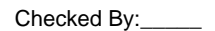
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 30° 160mph 30psf 3.5ft 7-10.rdb Page 20



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 30° 160mph 30psf 3.5ft 7-10.r Page 21



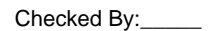
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	199.971	2	-.015	2	.064	1	0	10	0	4	0	15
30		min	-365.201	3	-.074	3	-.586	5	0	4	0	3	0	6
31	16	max	200.097	2	-.028	15	.064	1	0	10	0	4	0	15
32		min	-365.106	3	-.114	4	-.701	5	0	4	0	3	0	6
33	17	max	200.223	2	-.04	15	.064	1	0	10	0	4	0	15
34		min	-365.012	3	-.165	4	-.815	5	0	4	0	3	0	6
35	18	max	200.349	2	-.052	15	.064	1	0	10	0	9	0	15
36		min	-364.918	3	-.216	4	-.93	5	0	4	0	3	0	6
37	19	max	200.475	2	-.064	15	.064	1	0	10	0	9	0	15
38		min	-364.823	3	-.267	4	-1.044	5	0	4	0	3	0	6
39	M3	1	max	197.13	2	1.757	.011	10	0	5	0	4	0	6
40		min	-182.966	3	.412	15	-1.319	4	0	1	0	10	0	15
41	2	max	197.06	2	1.58	6	.011	10	0	5	0	1	0	2
42		min	-183.018	3	.37	15	-1.186	4	0	1	0	10	0	12
43	3	max	196.991	2	1.403	6	.011	10	0	5	0	1	0	2
44		min	-183.07	3	.329	15	-1.052	4	0	1	0	5	0	3
45	4	max	196.922	2	1.226	6	.011	10	0	5	0	1	0	15
46		min	-183.122	3	.287	15	-.918	4	0	1	0	5	0	4
47	5	max	196.852	2	1.049	6	.011	10	0	5	0	1	0	15
48		min	-183.174	3	.246	15	-.785	4	0	1	0	5	0	4
49	6	max	196.783	2	.873	6	.011	10	0	5	0	1	0	15
50		min	-183.226	3	.204	15	-.651	4	0	1	0	5	0	4
51	7	max	196.714	2	.696	6	.011	10	0	5	0	1	0	15
52		min	-183.278	3	.163	15	-.517	4	0	1	0	5	0	4
53	8	max	196.644	2	.519	6	.011	10	0	5	0	1	0	15
54		min	-183.33	3	.121	15	-.384	4	0	1	0	5	-.001	4
55	9	max	196.575	2	.342	6	.011	10	0	5	0	1	0	15
56		min	-183.382	3	.08	15	-.25	4	0	1	0	5	-.001	4
57	10	max	196.506	2	.165	6	.011	10	0	5	0	1	0	15
58		min	-183.434	3	.038	15	-.116	4	0	1	0	5	-.001	4
59	11	max	196.436	2	.02	2	.043	5	0	5	0	1	0	15
60		min	-183.486	3	-.039	3	-.099	1	0	1	0	5	-.001	4
61	12	max	196.367	2	-.045	15	.177	5	0	5	0	1	0	15
62		min	-183.538	3	-.188	4	-.099	1	0	1	0	5	-.001	4
63	13	max	196.298	2	-.087	15	.31	5	0	5	0	1	0	15
64		min	-183.59	3	-.365	4	-.099	1	0	1	0	5	-.001	4
65	14	max	196.228	2	-.128	15	.444	5	0	5	0	1	0	15
66		min	-183.642	3	-.542	4	-.099	1	0	1	0	5	-.001	4
67	15	max	196.159	2	-.17	15	.578	5	0	5	0	9	0	15
68		min	-183.694	3	-.719	4	-.099	1	0	1	0	5	0	4
69	16	max	196.09	2	-.211	15	.711	5	0	5	0	9	0	15
70		min	-183.746	3	-.896	4	-.099	1	0	1	0	5	0	4
71	17	max	196.02	2	-.253	15	.845	5	0	5	0	10	0	15
72		min	-183.798	3	-1.073	4	-.099	1	0	1	0	4	0	4
73	18	max	195.951	2	-.295	15	.979	5	0	5	0	10	0	15
74		min	-183.85	3	-1.249	4	-.099	1	0	1	0	4	0	4
75	19	max	195.882	2	-.336	15	1.112	5	0	5	0	5	0	1
76		min	-183.902	3	-1.426	4	-.099	1	0	1	0	1	0	1
77	M4	1	max	245.034	1	0	.058	10	0	1	0	5	0	1
78		min	3.061	12	0	1	-13.426	4	0	1	0	2	0	1
79	2	max	245.099	1	0	1	.058	10	0	1	0	10	0	1
80		min	3.093	12	0	1	-13.483	4	0	1	-.001	4	0	1
81	3	max	245.163	1	0	1	.058	10	0	1	0	10	0	1
82		min	3.125	12	0	1	-13.539	4	0	1	-.002	4	0	1
83	4	max	245.228	1	0	1	.058	10	0	1	0	10	0	1
84		min	3.158	12	0	1	-13.595	4	0	1	-.004	4	0	1
85	5	max	245.293	1	0	1	.058	10	0	1	0	10	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	607.62	2	.046	2	.011	9	0	3	0	4	0	12
144		min	-1066.637	3	-.15	3	-.619	5	0	5	0	3	0	2
145	16	max	607.746	2	.006	2	.011	9	0	3	0	4	0	12
146		min	-1066.543	3	-.18	3	-.733	5	0	5	0	3	0	2
147	17	max	607.872	2	-.034	2	.011	9	0	3	0	4	0	12
148		min	-1066.448	3	-.21	3	-.848	5	0	5	0	3	0	2
149	18	max	607.998	2	-.063	15	.011	9	0	3	0	4	0	3
150		min	-1066.354	3	-.24	3	-.962	5	0	5	0	3	0	2
151	19	max	608.124	2	-.075	15	.011	9	0	3	0	9	0	3
152		min	-1066.26	3	-.284	4	-1.076	5	0	5	0	3	0	2
153	M7	1	max	587.349	2	1.776	.042	3	0	9	0	4	0	2
154		min	-485.767	3	.424	15	-1.321	4	0	3	0	3	0	3
155	2	max	587.28	2	1.6	4	.042	3	0	9	0	4	0	2
156		min	-485.819	3	.383	15	-1.187	4	0	3	0	3	0	3
157	3	max	587.21	2	1.423	4	.042	3	0	9	0	9	0	2
158		min	-485.871	3	.341	15	-1.054	4	0	3	0	3	0	3
159	4	max	587.141	2	1.246	4	.042	3	0	9	0	9	0	2
160		min	-485.923	3	.3	15	-.92	4	0	3	0	3	0	3
161	5	max	587.072	2	1.069	4	.042	3	0	9	0	9	0	15
162		min	-485.975	3	.258	15	-.786	4	0	3	0	5	0	3
163	6	max	587.002	2	.892	4	.042	3	0	9	0	9	0	15
164		min	-486.027	3	.217	15	-.653	4	0	3	0	5	0	6
165	7	max	586.933	2	.715	4	.042	3	0	9	0	9	0	15
166		min	-486.079	3	.175	15	-.519	4	0	3	0	5	0	6
167	8	max	586.864	2	.539	4	.042	3	0	9	0	9	0	15
168		min	-486.131	3	.134	15	-.385	4	0	3	0	5	-.001	6
169	9	max	586.794	2	.362	4	.042	3	0	9	0	9	0	15
170		min	-486.183	3	.081	12	-.252	4	0	3	0	5	-.001	6
171	10	max	586.725	2	.21	2	.042	3	0	9	0	9	0	15
172		min	-486.235	3	.009	3	-.118	4	0	3	0	5	-.001	6
173	11	max	586.656	2	.072	2	.042	3	0	9	0	9	0	15
174		min	-486.287	3	-.095	3	-.003	9	0	3	-.001	5	-.001	6
175	12	max	586.587	2	-.033	15	.151	5	0	9	0	9	0	15
176		min	-486.339	3	-.198	3	-.003	9	0	3	0	5	-.001	6
177	13	max	586.517	2	-.074	15	.284	5	0	9	0	9	0	15
178		min	-486.391	3	-.346	6	-.003	9	0	3	0	5	-.001	6
179	14	max	586.448	2	-.116	15	.418	5	0	9	0	9	0	15
180		min	-486.443	3	-.523	6	-.003	9	0	3	0	5	-.001	6
181	15	max	586.379	2	-.157	15	.552	5	0	9	0	9	0	15
182		min	-486.495	3	-.7	6	-.003	9	0	3	0	5	0	6
183	16	max	586.309	2	-.199	15	.685	5	0	9	0	9	0	15
184		min	-486.547	3	-.877	6	-.003	9	0	3	0	5	0	6
185	17	max	586.24	2	-.241	15	.819	5	0	9	0	9	0	15
186		min	-486.599	3	-1.054	6	-.003	9	0	3	0	5	0	6
187	18	max	586.171	2	-.282	15	.953	5	0	9	0	9	0	15
188		min	-486.651	3	-1.23	6	-.003	9	0	3	0	3	0	6
189	19	max	586.101	2	-.324	15	1.086	5	0	9	0	9	0	1
190		min	-486.703	3	-1.407	6	-.003	9	0	3	0	3	0	1
191	M8	1	max	699.686	2	0	.085	9	0	1	0	4	0	1
192		min	-45.814	3	0	1	-13.691	4	0	1	0	3	0	1
193	2	max	699.75	2	0	1	.085	9	0	1	0	9	0	1
194		min	-45.765	3	0	1	-13.747	4	0	1	-.001	4	0	1
195	3	max	699.815	2	0	1	.085	9	0	1	0	9	0	1
196		min	-45.717	3	0	1	-13.803	4	0	1	-.002	4	0	1
197	4	max	699.88	2	0	1	.085	9	0	1	0	9	0	1
198		min	-45.668	3	0	1	-13.859	4	0	1	-.004	4	0	1
199	5	max	699.944	2	0	1	.085	9	0	1	0	9	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	201.147	2	.01	5	.005	10	0	1	0	5	0	15
258		min	-274.156	3	-.065	3	-.53	4	-.001	5	0	3	0	4
259	16	max	201.272	2	-.005	15	.005	10	0	1	0	5	0	15
260		min	-274.062	3	-.095	3	-.644	4	-.001	5	0	3	0	4
261	17	max	201.398	2	-.017	15	.005	10	0	1	0	5	0	12
262		min	-273.968	3	-.132	6	-.759	4	-.001	5	0	3	0	4
263	18	max	201.524	2	-.029	15	.005	10	0	1	0	5	0	12
264		min	-273.873	3	-.183	6	-.873	4	-.001	5	0	3	0	4
265	19	max	201.65	2	-.041	15	.005	10	0	1	0	5	0	12
266		min	-273.779	3	-.234	6	-.988	4	-.001	5	0	3	0	4
267	M11	1	max	196.733	2	1.746	.099	1	0	4	0	5	0	2
268		min	-183.933	3	.405	15	-1.27	5	0	10	0	1	0	15
269	2	max	196.664	2	1.569	6	.099	1	0	4	0	3	0	2
270		min	-183.985	3	.363	15	-1.136	5	0	10	0	1	0	15
271	3	max	196.595	2	1.393	6	.099	1	0	4	0	3	0	2
272		min	-184.037	3	.321	15	-1.002	5	0	10	0	1	0	3
273	4	max	196.525	2	1.216	6	.099	1	0	4	0	3	0	15
274		min	-184.089	3	.28	15	-.869	5	0	10	0	1	0	4
275	5	max	196.456	2	1.039	6	.099	1	0	4	0	3	0	15
276		min	-184.141	3	.238	15	-.735	5	0	10	0	4	0	4
277	6	max	196.387	2	.862	6	.099	1	0	4	0	3	0	15
278		min	-184.193	3	.197	15	-.601	5	0	10	0	4	0	4
279	7	max	196.317	2	.685	6	.099	1	0	4	0	3	0	15
280		min	-184.245	3	.155	15	-.468	5	0	10	0	4	0	4
281	8	max	196.248	2	.508	6	.099	1	0	4	0	3	0	15
282		min	-184.297	3	.114	15	-.334	5	0	10	0	4	-.001	4
283	9	max	196.179	2	.332	6	.099	1	0	4	0	3	0	15
284		min	-184.349	3	.072	15	-.2	5	0	10	0	4	-.001	4
285	10	max	196.109	2	.158	2	.099	1	0	4	0	3	0	15
286		min	-184.401	3	.031	15	-.067	5	0	10	0	4	-.001	4
287	11	max	196.04	2	.02	2	.099	1	0	4	0	3	0	15
288		min	-184.453	3	-.04	3	-.061	3	0	10	0	4	-.001	4
289	12	max	195.971	2	-.053	15	.227	4	0	4	0	3	0	15
290		min	-184.505	3	-.199	4	-.061	3	0	10	0	4	-.001	4
291	13	max	195.901	2	-.094	15	.361	4	0	4	0	3	0	15
292		min	-184.557	3	-.376	4	-.061	3	0	10	0	4	-.001	4
293	14	max	195.832	2	-.136	15	.494	4	0	4	0	3	0	15
294		min	-184.609	3	-.553	4	-.061	3	0	10	0	4	-.001	4
295	15	max	195.763	2	-.177	15	.628	4	0	4	0	3	0	15
296		min	-184.661	3	-.73	4	-.061	3	0	10	0	4	0	4
297	16	max	195.693	2	-.219	15	.762	4	0	4	0	3	0	15
298		min	-184.713	3	-.907	4	-.061	3	0	10	0	5	0	4
299	17	max	195.624	2	-.26	15	.895	4	0	4	0	3	0	15
300		min	-184.765	3	-1.084	4	-.061	3	0	10	0	5	0	4
301	18	max	195.555	2	-.302	15	1.029	4	0	4	0	3	0	15
302		min	-184.817	3	-1.26	4	-.061	3	0	10	0	10	0	4
303	19	max	195.485	2	-.344	15	1.163	4	0	4	0	4	0	1
304		min	-184.869	3	-1.437	4	-.061	3	0	10	0	10	0	1
305	M12	1	max	245.388	1	0	.464	3	0	1	0	4	0	1
306		min	-.657	15	0	1	-12.613	5	0	1	0	3	0	1
307	2	max	245.453	1	0	1	.464	3	0	1	0	1	0	1
308		min	-.638	15	0	1	-12.669	5	0	1	-.001	5	0	1
309	3	max	245.517	1	0	1	.464	3	0	1	0	1	0	1
310		min	-.618	15	0	1	-12.725	5	0	1	-.002	5	0	1
311	4	max	245.582	1	0	1	.464	3	0	1	0	1	0	1
312		min	-.598	15	0	1	-12.781	5	0	1	-.003	5	0	1
313	5	max	245.647	1	0	1	.464	3	0	1	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-579	15	0	1	-12.838	5	0	1	-.005	5	0	1
315	6	max	245.711	1	0	1	.464	3	0	1	0	1	0	1
316		min	-.559	15	0	1	-12.894	5	0	1	-.006	5	0	1
317	7	max	245.776	1	0	1	.464	3	0	1	0	1	0	1
318		min	-.54	15	0	1	-12.95	5	0	1	-.007	5	0	1
319	8	max	245.841	1	0	1	.464	3	0	1	0	1	0	1
320		min	-.52	15	0	1	-13.006	5	0	1	-.008	5	0	1
321	9	max	245.906	1	0	1	.464	3	0	1	0	1	0	1
322		min	-.501	15	0	1	-13.062	5	0	1	-.009	5	0	1
323	10	max	245.97	1	0	1	.464	3	0	1	0	1	0	1
324		min	-.481	15	0	1	-13.118	5	0	1	-.01	5	0	1
325	11	max	246.035	1	0	1	.464	3	0	1	0	1	0	1
326		min	-.462	15	0	1	-13.174	5	0	1	-.012	5	0	1
327	12	max	246.1	1	0	1	.464	3	0	1	0	1	0	1
328		min	-.442	15	0	1	-13.23	5	0	1	-.013	5	0	1
329	13	max	246.164	1	0	1	.464	3	0	1	0	1	0	1
330		min	-.423	15	0	1	-13.286	5	0	1	-.014	5	0	1
331	14	max	246.229	1	0	1	.464	3	0	1	0	3	0	1
332		min	-.403	15	0	1	-13.342	5	0	1	-.015	5	0	1
333	15	max	246.294	1	0	1	.464	3	0	1	0	3	0	1
334		min	-.384	15	0	1	-13.398	5	0	1	-.016	5	0	1
335	16	max	246.358	1	0	1	.464	3	0	1	0	3	0	1
336		min	-.364	15	0	1	-13.454	5	0	1	-.017	5	0	1
337	17	max	246.423	1	0	1	.464	3	0	1	0	3	0	1
338		min	-.345	15	0	1	-13.51	5	0	1	-.019	5	0	1
339	18	max	246.488	1	0	1	.464	3	0	1	0	3	0	1
340		min	-.325	15	0	1	-13.567	5	0	1	-.02	5	0	1
341	19	max	246.553	1	0	1	.464	3	0	1	0	3	0	1
342		min	-.306	15	0	1	-13.623	5	0	1	-.021	5	0	1
343	M1	1	max	64.647	1	345.815	3	1.378	10	0	.026	4	0	2
344		min	3.791	10	-218.968	2	-15.033	4	0	3	-.003	10	0	3
345	2	max	64.787	1	345.633	3	1.378	10	0	2	.023	4	.048	2
346		min	3.907	10	-219.21	2	-14.791	4	0	3	-.002	10	-.075	3
347	3	max	92.725	3	4.519	4	1.373	10	0	10	.019	4	.095	2
348		min	-20.763	2	-25.711	2	-13.511	4	0	1	-.002	10	-.149	3
349	4	max	92.829	3	4.209	4	1.373	10	0	10	.016	4	.1	2
350		min	-20.623	2	-25.953	2	-13.269	4	0	1	-.002	10	-.146	3
351	5	max	92.934	3	3.898	4	1.373	10	0	10	.014	4	.106	2
352		min	-20.484	2	-26.195	2	-13.027	4	0	1	-.002	10	-.143	3
353	6	max	93.039	3	3.588	4	1.373	10	0	10	.011	4	.112	2
354		min	-20.344	2	-26.437	2	-12.785	4	0	1	-.001	10	-.141	3
355	7	max	93.143	3	3.326	14	1.373	10	0	10	.008	4	.117	2
356		min	-20.205	2	-26.678	2	-12.543	4	0	1	0	10	-.138	3
357	8	max	93.248	3	3.089	14	1.373	10	0	10	.005	4	.123	2
358		min	-20.065	2	-26.92	2	-12.301	4	0	1	0	10	-.135	3
359	9	max	93.353	3	2.851	14	1.373	10	0	10	.003	3	.129	2
360		min	-19.925	2	-27.162	2	-12.113	1	0	1	0	10	-.132	3
361	10	max	93.458	3	2.613	14	1.373	10	0	10	.002	3	.135	2
362		min	-19.786	2	-27.404	2	-12.113	1	0	1	0	10	-.129	3
363	11	max	93.562	3	2.376	14	1.373	10	0	10	0	3	.141	2
364		min	-19.646	2	-27.646	2	-12.113	1	0	1	-.003	1	-.126	3
365	12	max	93.667	3	2.138	14	1.373	10	0	10	0	10	.147	2
366		min	-19.507	2	-27.888	2	-12.113	1	0	1	-.005	1	-.123	3
367	13	max	93.772	3	1.901	14	1.373	10	0	10	0	10	.153	2
368		min	-19.367	2	-28.129	2	-12.113	1	0	1	-.008	1	-.12	3
369	14	max	93.876	3	1.663	14	1.373	10	0	10	.001	10	.159	2
370		min	-19.227	2	-28.371	2	-12.113	1	0	1	-.01	1	-.117	3







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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	30.294	4	27.867	2	10.708	4	0	3	0	5	.231	3
486			min	-1.378	10	-48.565	3	-5.558	2	0	2	-.025	1	-.149	2
487		16	max	26.475	4	50.019	3	15.324	4	0	3	.005	5	.231	3
488			min	-1.378	10	-33.825	2	-1.718	10	0	2	-.023	1	-.147	2
489		17	max	22.655	4	148.602	3	31.615	1	0	3	.01	5	.192	3
490			min	-1.378	10	-95.517	2	.118	10	0	2	-.014	1	-.122	2
491		18	max	18.836	4	247.186	3	48.13	1	0	3	.016	4	.115	3
492			min	-1.378	10	-157.209	2	1.955	10	0	2	-.005	2	-.073	2
493		19	max	15.016	4	345.77	3	64.644	1	0	3	.026	4	0	2
494			min	-1.378	10	-218.901	2	3.791	10	0	2	-.003	10	0	3
495	M16	1	max	26.757	5	325.635	2	7.938	5	0	3	.024	1	0	2
496			min	-12.639	1	-166.445	3	-64.645	1	0	2	-.023	5	0	3
497		2	max	22.938	5	233.429	2	8.966	5	0	3	.004	9	.056	3
498			min	-12.639	1	-120.17	3	-48.131	1	0	2	-.02	5	-.109	2
499		3	max	19.118	5	141.224	2	9.995	5	0	3	0	3	.093	3
500			min	-12.639	1	-73.896	3	-31.616	1	0	2	-.019	4	-.182	2
501		4	max	15.299	5	49.018	2	11.023	5	0	3	-.001	12	.113	3
502			min	-12.639	1	-27.621	3	-15.102	1	0	2	-.023	1	-.219	2
503		5	max	11.479	5	18.653	3	12.051	5	0	3	-.003	12	.115	3
504			min	-12.639	1	-43.188	2	-5.457	3	0	2	-.025	1	-.22	2
505		6	max	7.66	5	64.928	3	17.927	1	0	3	-.001	10	.099	3
506			min	-12.639	1	-135.393	2	-4.49	3	0	2	-.021	1	-.185	2
507		7	max	3.84	5	111.202	3	34.441	1	0	3	.003	5	.064	3
508			min	-12.639	1	-227.599	2	-3.522	3	0	2	-.011	1	-.114	2
509		8	max	2.541	3	157.477	3	50.955	1	0	3	.009	4	.012	3
510			min	-12.639	1	-319.805	2	-2.555	3	0	2	-.009	3	-.008	2
511		9	max	2.541	3	203.751	3	67.47	1	0	3	.028	1	.134	2
512			min	-12.639	1	-412.01	2	-1.588	3	0	2	-.01	3	-.058	3
513		10	max	15.884	5	-6.27	15	83.984	1	0	14	.058	1	.313	2
514			min	-12.639	1	-504.216	2	-1.067	3	0	2	-.01	3	-.146	3
515		11	max	12.064	5	412.01	2	4.916	5	0	2	.028	1	.134	2
516			min	-12.639	1	-203.751	3	-67.47	1	0	3	-.009	5	-.058	3
517		12	max	8.245	5	319.805	2	5.945	5	0	2	.008	2	.012	3
518			min	-12.639	1	-157.477	3	-50.955	1	0	3	-.007	5	-.008	2
519		13	max	4.425	5	227.599	2	6.973	5	0	2	.001	10	.064	3
520			min	-12.639	1	-111.202	3	-34.441	1	0	3	-.011	1	-.114	2
521		14	max	1.441	10	135.393	2	8.001	5	0	2	0	15	.099	3
522			min	-12.639	1	-64.928	3	-17.927	1	0	3	-.021	1	-.185	2
523		15	max	1.441	10	43.188	2	9.453	4	0	2	.002	5	.115	3
524			min	-12.639	1	-18.653	3	-5.539	2	0	3	-.025	1	-.22	2
525		16	max	1.441	10	27.621	3	15.102	1	0	2	.006	5	.113	3
526			min	-12.639	1	-49.018	2	-1.705	10	0	3	-.023	1	-.219	2
527		17	max	1.441	10	73.896	3	31.616	1	0	2	.01	5	.093	3
528			min	-14.19	4	-141.224	2	.131	10	0	3	-.013	1	-.182	2
529		18	max	1.441	10	120.17	3	48.131	1	0	2	.016	4	.056	3
530			min	-18.01	4	-233.429	2	1.968	10	0	3	-.005	2	-.109	2
531		19	max	1.441	10	166.445	3	64.645	1	0	2	.026	4	0	2
532			min	-21.829	4	-325.635	2	3.804	10	0	3	-.003	10	0	5
533	M15	1	max	0	1	.737	3	.167	3	0	1	0	1	0	1
534			min	-132.763	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.655	3	.167	3	0	1	0	1	0	1
536			min	-132.833	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.573	3	.167	3	0	1	0	1	0	1
538			min	-132.904	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.491	3	.167	3	0	1	0	1	0	1
540			min	-132.974	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.409	3	.167	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-133.045	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.327	3	.167	3	0	1	0	1	0	1
544		min	-133.115	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.246	3	.167	3	0	1	0	3	0	1
546		min	-133.186	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.164	3	.167	3	0	1	0	3	0	1
548		min	-133.256	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.082	3	.167	3	0	1	0	3	0	1
550		min	-133.327	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.167	3	0	1	0	3	0	1
552		min	-133.397	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.167	3	0	1	0	3	0	1
554		min	-133.468	3	-.082	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.167	3	0	1	0	3	0	1
556		min	-133.538	3	-.164	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.167	3	0	1	0	3	0	1
558		min	-133.609	3	-.246	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.167	3	0	1	0	3	0	1
560		min	-133.679	3	-.327	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.167	3	0	1	0	3	0	1
562		min	-133.75	3	-.409	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.167	3	0	1	0	3	0	1
564		min	-133.82	3	-.491	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.167	3	0	1	0	3	0	1
566		min	-133.891	3	-.573	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.167	3	0	1	0	3	0	1
568		min	-133.961	3	-.655	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.167	3	0	1	0	3	0	1
570		min	-134.032	3	-.737	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.966	.335	4	0	3	0	3	0	1
572		min	-165.037	4	0	1	-.07	3	0	4	0	4	0	1
573	2	max	0	1	1.747	4	.301	4	0	3	0	3	0	1
574		min	-165.007	4	0	1	-.07	3	0	4	0	4	0	4
575	3	max	0	1	1.529	4	.267	4	0	3	0	3	0	1
576		min	-164.978	4	0	1	-.07	3	0	4	0	4	0	4
577	4	max	0	1	1.311	4	.233	4	0	3	0	3	0	1
578		min	-164.948	4	0	1	-.07	3	0	4	0	4	-.001	4
579	5	max	0	1	1.092	4	.199	4	0	3	0	3	0	1
580		min	-164.918	4	0	1	-.07	3	0	4	0	9	-.002	4
581	6	max	0	1	.874	4	.165	4	0	3	0	3	0	1
582		min	-164.889	4	0	1	-.07	3	0	4	0	9	-.002	4
583	7	max	0	1	.655	4	.131	4	0	3	0	3	0	1
584		min	-164.859	4	0	1	-.07	3	0	4	0	9	-.002	4
585	8	max	0	1	.437	4	.097	4	0	3	0	5	0	1
586		min	-164.829	4	0	1	-.07	3	0	4	0	9	-.002	4
587	9	max	0	1	.218	4	.063	4	0	3	0	5	0	1
588		min	-164.8	4	0	1	-.07	3	0	4	0	9	-.002	4
589	10	max	0	1	0	1	.029	4	0	3	0	5	0	1
590		min	-164.77	4	0	1	-.07	3	0	4	0	9	-.002	4
591	11	max	0	1	0	1	.012	9	0	3	0	5	0	1
592		min	-164.741	4	-.218	4	-.07	3	0	4	0	9	-.002	4
593	12	max	0	1	0	1	.012	9	0	3	0	5	0	1
594		min	-164.711	4	-.437	4	-.07	3	0	4	0	9	-.002	4
595	13	max	0	1	0	1	.012	9	0	3	0	5	0	1
596		min	-164.681	4	-.655	4	-.076	5	0	4	0	3	-.002	4
597	14	max	0	1	0	1	.012	9	0	3	0	5	0	1
598		min	-164.652	4	-.874	4	-.11	5	0	4	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.003	9	0	1	.012	9	0	3	0	5	0	1
600		min	-164.622	4	-1.092	4	-.144	5	0	4	0	3	-.002	4
601	16	max	.081	9	0	1	.012	9	0	3	0	5	0	1
602		min	-164.667	5	-1.311	4	-.178	5	0	4	0	3	-.001	4
603	17	max	.159	9	0	1	.012	9	0	3	0	9	0	1
604		min	-164.714	5	-1.529	4	-.212	5	0	4	0	3	0	4
605	18	max	.238	9	0	1	.012	9	0	3	0	9	0	1
606		min	-164.761	5	-1.747	4	-.246	5	0	4	0	3	0	4
607	19	max	.316	9	0	1	.012	9	0	3	0	9	0	1
608		min	-164.808	5	-1.966	4	-.28	5	0	4	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.01	2	.002	9	7.636e-4	5	NC	3	NC	1	
2			min	-.004	3	-.01	3	-.009	5	-2.216e-4	3	4108.037	2	NC	1	
3			2	max	.002	2	.009	2	.002	9	7.843e-4	5	NC	3	NC	1
4				min	-.003	3	-.009	3	-.009	5	-2.098e-4	3	4481.036	2	NC	1
5			3	max	.002	2	.008	2	.001	9	8.05e-4	5	NC	3	NC	1
6				min	-.003	3	-.009	3	-.009	5	-1.98e-4	3	4924.187	2	NC	1
7			4	max	.002	2	.007	2	.001	9	8.256e-4	5	NC	1	NC	1
8				min	-.003	3	-.008	3	-.008	5	-1.861e-4	3	5454.303	2	NC	1
9			5	max	.002	2	.006	2	.001	9	8.463e-4	5	NC	1	NC	1
10				min	-.003	3	-.008	3	-.008	5	-1.743e-4	3	6093.759	2	NC	1
11			6	max	.001	2	.006	2	.001	9	8.67e-4	5	NC	1	NC	1
12				min	-.003	3	-.008	3	-.008	5	-1.625e-4	3	6872.814	2	NC	1
13			7	max	.001	2	.005	2	0	9	8.877e-4	5	NC	1	NC	1
14				min	-.002	3	-.007	3	-.007	5	-1.507e-4	3	7833.166	2	NC	1
15			8	max	.001	2	.004	2	0	9	9.084e-4	5	NC	1	NC	1
16				min	-.002	3	-.007	3	-.007	5	-1.389e-4	3	9033.549	2	NC	1
17			9	max	.001	2	.004	2	0	9	9.29e-4	5	NC	1	NC	1
18				min	-.002	3	-.006	3	-.007	5	-1.27e-4	3	NC	1	NC	1
19			10	max	0	2	.003	2	0	9	9.497e-4	5	NC	1	NC	1
20				min	-.002	3	-.006	3	-.006	5	-1.152e-4	3	NC	1	NC	1
21		11	max	0	2	.003	2	0	9	9.704e-4	5	NC	1	NC	1	
22			min	-.002	3	-.005	3	-.006	5	-1.034e-4	3	NC	1	NC	1	
23		12	max	0	2	.002	2	0	9	9.911e-4	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.005	5	-9.401e-5	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	9	1.012e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.004	5	-8.474e-5	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	9	1.032e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.004	5	-7.547e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	9	1.053e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.003	5	-6.62e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	9	1.074e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-5.693e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	9	1.095e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-4.765e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	9	1.115e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-3.838e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.136e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.038e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.436e-5	9	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.352e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.003	5	1.982e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	9	-5.373e-4	5	NC	1	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.005	5	2.583e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.393e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.008	5	3.183e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	9	-5.414e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.011	4	3.784e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.435e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.014	4	4.385e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.456e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.016	4	4.985e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-5.476e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.019	4	5.586e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	10	-5.497e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.021	4	6.186e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-5.518e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.024	4	6.787e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-5.539e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.026	4	7.387e-5	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-5.559e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.028	4	7.988e-5	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-5.58e-4	5	NC	1	NC	1
63		13	max	.001	3	.004	2	.03	4	8.589e-5	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-5.601e-4	5	NC	1	NC	1
65		14	max	.002	3	.004	2	.032	4	9.189e-5	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-5.622e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.034	4	9.79e-5	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-5.643e-4	5	8893.752	2	NC	1
69		16	max	.002	3	.006	2	.036	4	1.039e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-5.663e-4	5	7543.653	2	NC	1
71		17	max	.002	3	.007	2	.038	4	1.099e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-5.684e-4	5	6497.074	2	NC	1
73		18	max	.002	3	.008	2	.04	4	1.159e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-5.705e-4	5	5676.903	2	NC	1
75		19	max	.002	3	.009	2	.041	4	1.219e-4	1	NC	3	NC	1
76			min	-.002	2	-.009	3	0	10	-5.726e-4	5	5028.646	2	NC	1
77	M4	1	max	.001	1	.011	2	0	10	2.802e-3	5	NC	1	NC	1
78			min	0	12	-.01	3	-.044	4	-1.396e-4	1	NC	1	443.697	4
79		2	max	.001	1	.01	2	0	10	2.802e-3	5	NC	1	NC	1
80			min	0	12	-.009	3	-.04	4	-1.396e-4	1	NC	1	483.615	4
81		3	max	.001	1	.01	2	0	10	2.802e-3	5	NC	1	NC	1
82			min	0	12	-.009	3	-.036	4	-1.396e-4	1	NC	1	531.116	4
83		4	max	0	1	.009	2	0	10	2.802e-3	5	NC	1	NC	1
84			min	0	12	-.008	3	-.033	4	-1.396e-4	1	NC	1	588.198	4
85		5	max	0	1	.008	2	0	10	2.802e-3	5	NC	1	NC	1
86			min	0	12	-.007	3	-.029	4	-1.396e-4	1	NC	1	657.579	4
87		6	max	0	1	.008	2	0	10	2.802e-3	5	NC	1	NC	1
88			min	0	12	-.007	3	-.026	4	-1.396e-4	1	NC	1	743.044	4
89		7	max	0	1	.007	2	0	10	2.802e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.023	4	-1.396e-4	1	NC	1	849.978	4
91		8	max	0	1	.007	2	0	10	2.802e-3	5	NC	1	NC	1
92			min	0	12	-.006	3	-.02	4	-1.396e-4	1	NC	1	986.268	4
93		9	max	0	1	.006	2	0	10	2.802e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.017	4	-1.396e-4	1	NC	1	1163.843	4
95		10	max	0	1	.005	2	0	10	2.802e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.014	4	-1.396e-4	1	NC	1	1401.451	4
97		11	max	0	1	.005	2	0	10	2.802e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.011	4	-1.396e-4	1	NC	1	1729.969	4
99		12	max	0	1	.004	2	0	10	2.802e-3	5	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.006	4	4.224e-6	9	NC	1	NC	1
158			min	0	2	-.004	3	0	9	-5.46e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.009	4	3.555e-6	9	NC	1	NC	1
160			min	-.001	2	-.006	3	0	9	-5.39e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.011	4	2.885e-6	9	NC	1	NC	1
162			min	-.001	2	-.007	3	0	9	-5.321e-4	4	9342.157	2	NC	1
163		6	max	.002	3	.006	2	.014	4	2.417e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	9	-5.251e-4	4	7482.063	2	NC	1
165		7	max	.002	3	.007	2	.017	4	5.091e-5	3	NC	1	NC	1
166			min	-.002	2	-.011	3	0	9	-5.181e-4	4	6207.992	2	NC	1
167		8	max	.002	3	.009	2	.02	4	7.766e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	-5.111e-4	4	5272.694	2	NC	1
169		9	max	.002	3	.01	2	.022	4	1.044e-4	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	9	-5.041e-4	4	4553.012	2	NC	1
171		10	max	.003	3	.012	2	.025	4	1.311e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	9	-4.971e-4	4	3980.672	2	NC	1
173		11	max	.003	3	.013	2	.027	4	1.579e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	-4.902e-4	4	3514.633	2	NC	1
175		12	max	.003	3	.015	2	.029	4	1.846e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	9	-4.832e-4	4	3128.582	2	NC	1
177		13	max	.004	3	.016	2	.031	4	2.114e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	0	9	-4.762e-4	4	2804.718	2	NC	1
179		14	max	.004	3	.018	2	.033	4	2.381e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-4.692e-4	4	2530.451	2	NC	1
181		15	max	.004	3	.02	2	.035	4	2.649e-4	3	NC	3	NC	1
182			min	-.005	2	-.021	3	0	9	-4.622e-4	4	2296.533	2	NC	1
183		16	max	.005	3	.022	2	.037	4	2.916e-4	3	NC	3	NC	1
184			min	-.006	2	-.022	3	0	9	-4.552e-4	4	2095.969	2	NC	1
185		17	max	.005	3	.024	2	.039	4	3.184e-4	3	NC	3	NC	1
186			min	-.006	2	-.023	3	0	9	-4.483e-4	4	1923.333	2	NC	1
187		18	max	.005	3	.026	2	.041	4	3.451e-4	3	NC	3	NC	1
188			min	-.006	2	-.024	3	0	9	-4.413e-4	4	1774.341	2	NC	1
189		19	max	.006	3	.028	2	.043	4	3.718e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	9	-4.343e-4	4	1645.561	2	NC	1
191	M8	1	max	.003	2	.033	2	0	9	2.667e-3	4	NC	1	NC	1
192			min	0	3	-.028	3	-.044	4	-2.688e-4	3	NC	1	435.384	4
193		2	max	.003	2	.031	2	0	9	2.667e-3	4	NC	1	NC	1
194			min	0	3	-.026	3	-.041	4	-2.688e-4	3	NC	1	474.556	4
195		3	max	.003	2	.03	2	0	9	2.667e-3	4	NC	1	NC	1
196			min	0	3	-.025	3	-.037	4	-2.688e-4	3	NC	1	521.17	4
197		4	max	.003	2	.028	2	0	9	2.667e-3	4	NC	1	NC	1
198			min	0	3	-.023	3	-.033	4	-2.688e-4	3	NC	1	577.185	4
199		5	max	.003	2	.026	2	0	9	2.667e-3	4	NC	1	NC	1
200			min	0	3	-.021	3	-.03	4	-2.688e-4	3	NC	1	645.272	4
201		6	max	.002	2	.024	2	0	9	2.667e-3	4	NC	1	NC	1
202			min	0	3	-.02	3	-.027	4	-2.688e-4	3	NC	1	729.142	4
203		7	max	.002	2	.022	2	0	9	2.667e-3	4	NC	1	NC	1
204			min	0	3	-.018	3	-.023	4	-2.688e-4	3	NC	1	834.082	4
205		8	max	.002	2	.02	2	0	9	2.667e-3	4	NC	1	NC	1
206			min	0	3	-.017	3	-.02	4	-2.688e-4	3	NC	1	967.831	4
207		9	max	.002	2	.018	2	0	9	2.667e-3	4	NC	1	NC	1
208			min	0	3	-.015	3	-.017	4	-2.688e-4	3	NC	1	1142.096	4
209		10	max	.002	2	.017	2	0	9	2.667e-3	4	NC	1	NC	1
210			min	0	3	-.014	3	-.014	4	-2.688e-4	3	NC	1	1375.277	4
211		11	max	.001	2	.015	2	0	9	2.667e-3	4	NC	1	NC	1
212			min	0	3	-.012	3	-.011	4	-2.688e-4	3	NC	1	1697.676	4
213		12	max	.001	2	.013	2	0	9	2.667e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.717e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.221e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.007	4	3.137e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-5.556e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.009	4	5.566e-6	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-5.89e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.012	4	4.903e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-6.224e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.014	4	5.568e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-6.559e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.016	5	6.232e-6	10	NC	1	NC	1
282			min	0	2	-.006	3	-.003	3	-6.893e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.019	5	6.897e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-7.227e-4	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.021	5	7.562e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-7.561e-4	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.023	5	8.226e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-7.896e-4	4	NC	1	NC	1
289		12	max	.001	3	.003	2	.025	5	8.891e-6	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.003	3	-8.23e-4	4	NC	1	NC	1
291		13	max	.001	3	.004	2	.027	5	9.556e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-8.564e-4	4	NC	1	NC	1
293		14	max	.002	3	.004	2	.029	5	1.022e-5	10	NC	1	NC	1
294			min	-.002	2	-.008	3	-.003	3	-8.899e-4	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.03	5	1.089e-5	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.003	3	-9.233e-4	4	8904.909	2	NC	1
297		16	max	.002	3	.006	2	.032	5	1.155e-5	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	3	-9.567e-4	4	7552.214	2	NC	1
299		17	max	.002	3	.007	2	.034	5	1.221e-5	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.003	3	-9.902e-4	4	6503.82	2	NC	1
301		18	max	.002	3	.008	2	.036	5	1.288e-5	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.003	3	-1.024e-3	4	5682.354	2	NC	1
303		19	max	.002	3	.009	2	.038	5	1.354e-5	10	NC	3	NC	1
304			min	-.002	2	-.009	3	-.002	3	-1.057e-3	4	5033.159	2	NC	1
305	M12	1	max	.001	1	.011	2	.001	1	3.221e-3	4	NC	1	NC	1
306			min	0	15	-.01	3	-.041	5	-1.598e-5	10	NC	1	471.579	5
307		2	max	.001	1	.01	2	.001	1	3.221e-3	4	NC	1	NC	1
308			min	0	15	-.009	3	-.038	5	-1.598e-5	10	NC	1	513.993	5
309		3	max	.001	1	.01	2	.001	3	3.221e-3	4	NC	1	NC	1
310			min	0	15	-.009	3	-.034	5	-1.598e-5	10	NC	1	564.464	5
311		4	max	0	1	.009	2	.001	3	3.221e-3	4	NC	1	NC	1
312			min	0	15	-.008	3	-.031	5	-1.598e-5	10	NC	1	625.112	5
313		5	max	0	1	.008	2	0	3	3.221e-3	4	NC	1	NC	1
314			min	0	15	-.008	3	-.028	5	-1.598e-5	10	NC	1	698.827	5
315		6	max	0	1	.008	2	0	3	3.221e-3	4	NC	1	NC	1
316			min	0	15	-.007	3	-.024	5	-1.598e-5	10	NC	1	789.628	5
317		7	max	0	1	.007	2	0	3	3.221e-3	4	NC	1	NC	1
318			min	0	15	-.006	3	-.021	5	-1.598e-5	10	NC	1	903.236	5
319		8	max	0	1	.007	2	0	3	3.221e-3	4	NC	1	NC	1
320			min	0	15	-.006	3	-.018	5	-1.598e-5	10	NC	1	1048.031	5
321		9	max	0	1	.006	2	0	3	3.221e-3	4	NC	1	NC	1
322			min	0	15	-.005	3	-.016	5	-1.598e-5	10	NC	1	1236.682	5
323		10	max	0	1	.005	2	0	3	3.221e-3	4	NC	1	NC	1
324			min	0	15	-.005	3	-.013	5	-1.598e-5	10	NC	1	1489.107	5
325		11	max	0	1	.005	2	0	3	3.221e-3	4	NC	1	NC	1
326			min	0	15	-.004	3	-.011	5	-1.598e-5	10	NC	1	1838.104	5
327		12	max	0	1	.004	2	0	3	3.221e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.008	5	-1.598e-5	10	NC	1	2340.879	5
329		max	0	1	.004	2	0	3	3.221e-3	4	NC	1	NC	1
330		min	0	15	-.003	3	-.006	5	-1.598e-5	10	NC	1	3104.727	5
331		max	0	1	.003	2	0	3	3.221e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.004	5	-1.598e-5	10	NC	1	4350.941	5
333		max	0	1	.002	2	0	3	3.221e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.003	5	-1.598e-5	10	NC	1	6599.37	5
335		max	0	1	.002	2	0	3	3.221e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.002	5	-1.598e-5	10	NC	1	NC	1
337		max	0	1	.001	2	0	3	3.221e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	0	5	-1.598e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	3	3.221e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	-1.598e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.221e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	-1.598e-5	10	NC	1	NC	1
343	M1	max	.009	3	.025	3	.006	5	4.89e-3	2	NC	1	NC	1
344		min	-.009	2	-.021	2	0	9	-7.219e-3	3	NC	1	NC	1
345		max	.009	3	.015	3	.007	5	2.42e-3	2	NC	4	NC	1
346		min	-.009	2	-.012	2	-.001	9	-3.549e-3	3	4646.666	3	NC	1
347		max	.009	3	.005	3	.009	5	2.486e-4	5	NC	4	NC	1
348		min	-.009	2	-.004	2	-.002	9	-8.507e-5	1	2406.75	3	NC	1
349		max	.009	3	.003	2	.011	5	2.48e-4	5	NC	4	NC	1
350		min	-.009	2	-.003	3	-.002	9	-7.129e-5	9	1714.528	3	8822.032	5
351		max	.009	3	.01	2	.013	5	2.475e-4	5	NC	4	NC	1
352		min	-.009	2	-.009	3	-.002	9	-5.838e-5	9	1384.384	3	6239.393	5
353		max	.009	3	.015	2	.015	5	2.47e-4	5	NC	4	NC	1
354		min	-.009	2	-.015	3	-.002	9	-4.547e-5	9	1200.074	3	4750.03	5
355		max	.009	3	.019	2	.018	5	2.465e-4	5	NC	4	NC	1
356		min	-.009	2	-.019	3	-.002	9	-3.257e-5	9	1090.926	3	3796.266	5
357		max	.009	3	.022	2	.021	5	2.46e-4	5	NC	4	NC	1
358		min	-.009	2	-.022	3	-.001	9	-1.966e-5	9	1022.673	2	3141.937	5
359		max	.009	3	.025	2	.023	5	2.455e-4	5	NC	4	NC	1
360		min	-.009	2	-.024	3	0	9	-6.755e-6	9	970.361	2	2670.62	5
361		max	.008	3	.025	2	.026	5	2.492e-4	4	NC	4	NC	1
362		min	-.009	2	-.024	3	0	9	-2.078e-6	10	945.219	2	2304.678	4
363		max	.008	3	.025	2	.029	4	2.531e-4	4	NC	4	NC	1
364		min	-.009	2	-.023	3	0	10	-3.892e-6	10	944.374	2	2023.453	4
365		max	.008	3	.023	2	.031	4	2.569e-4	4	NC	4	NC	1
366		min	-.009	2	-.021	3	0	10	-5.706e-6	10	968.792	2	1805.925	4
367		max	.008	3	.02	2	.034	4	2.608e-4	4	NC	4	NC	1
368		min	-.009	2	-.018	3	0	10	-7.52e-6	10	1023.846	2	1634.874	4
369		max	.008	3	.016	2	.037	4	2.647e-4	4	NC	4	NC	1
370		min	-.009	2	-.014	3	0	10	-9.333e-6	10	1122.084	2	1498.781	4
371		max	.008	3	.01	2	.039	4	2.686e-4	4	NC	4	NC	1
372		min	-.009	2	-.009	3	0	10	-1.115e-5	10	1291.189	2	1389.712	4
373		max	.008	3	.003	2	.041	4	4.248e-4	4	NC	4	NC	1
374		min	-.009	2	-.003	3	0	10	-1.249e-5	10	1599.223	2	1302.077	4
375		max	.008	3	.005	3	.044	4	4.208e-3	4	NC	4	NC	1
376		min	-.009	2	-.006	2	0	10	-2.691e-6	10	2266.745	2	1231.97	4
377		max	.008	3	.013	3	.045	4	3.511e-3	2	NC	4	NC	1
378		min	-.009	2	-.017	2	0	10	-1.937e-3	3	4394.162	2	1176.282	4
379		max	.008	3	.021	3	.047	4	7.086e-3	2	NC	1	NC	1
380		min	-.009	2	-.028	2	0	9	-3.995e-3	3	NC	1	1134.209	4
381	M5	max	.025	3	.076	3	.005	5	1.99e-5	4	NC	1	NC	1
382		min	-.027	2	-.063	2	0	9	7.827e-8	11	NC	1	NC	1
383		max	.025	3	.045	3	.007	5	1.485e-4	3	NC	4	NC	1
384		min	-.027	2	-.037	2	0	9	-8.196e-6	9	1525.317	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.025	3	.016	3	.009	5	2.82e-4	3	NC	5	NC	1
386		min	-.027	2	-.012	2	0	9	-1.633e-5	9	790.617	3	NC	1
387	4	max	.025	3	.011	2	.011	5	2.722e-4	3	NC	5	NC	1
388		min	-.027	2	-.009	3	0	9	-1.548e-5	9	564.175	3	NC	1
389	5	max	.025	3	.03	2	.013	5	2.625e-4	3	NC	5	NC	1
390		min	-.027	2	-.029	3	0	9	-1.464e-5	9	456.347	3	8773.676	3
391	6	max	.024	3	.046	2	.016	5	2.528e-4	3	NC	5	NC	1
392		min	-.027	2	-.046	3	0	9	-1.379e-5	9	396.303	3	7929.802	3
393	7	max	.024	3	.059	2	.019	5	2.576e-4	5	NC	5	NC	1
394		min	-.027	2	-.058	3	0	9	-1.295e-5	9	360.775	2	7548.703	3
395	8	max	.024	3	.069	2	.022	5	2.654e-4	5	NC	5	NC	1
396		min	-.027	2	-.067	3	0	9	-1.21e-5	9	332.378	2	7477.17	3
397	9	max	.024	3	.076	2	.024	5	2.731e-4	5	NC	5	NC	1
398		min	-.027	2	-.071	3	0	9	-1.126e-5	9	315.355	2	7658.6	3
399	10	max	.024	3	.078	2	.027	4	2.808e-4	5	NC	5	NC	1
400		min	-.027	2	-.073	3	0	9	-1.041e-5	9	307.176	2	8088.534	3
401	11	max	.024	3	.077	2	.03	4	2.885e-4	5	NC	5	NC	1
402		min	-.027	2	-.07	3	0	9	-9.567e-6	9	306.906	2	8804.146	3
403	12	max	.024	3	.072	2	.033	4	2.962e-4	5	NC	5	NC	1
404		min	-.027	2	-.064	3	0	9	-8.722e-6	9	314.855	2	9892.287	3
405	13	max	.024	3	.063	2	.036	4	3.041e-4	4	NC	5	NC	1
406		min	-.027	2	-.055	3	0	9	-7.877e-6	9	332.773	2	NC	1
407	14	max	.024	3	.05	2	.038	4	3.122e-4	4	NC	5	NC	1
408		min	-.027	2	-.043	3	0	9	-7.033e-6	9	364.738	2	NC	1
409	15	max	.024	3	.031	2	.041	4	3.203e-4	4	NC	5	NC	1
410		min	-.027	2	-.027	3	0	9	-6.188e-6	9	419.75	2	NC	1
411	16	max	.024	3	.009	2	.043	4	4.785e-4	4	NC	5	NC	1
412		min	-.027	2	-.008	3	0	9	-6.007e-6	9	519.928	2	NC	1
413	17	max	.024	3	.014	3	.044	4	4.21e-3	4	NC	5	NC	1
414		min	-.027	2	-.019	2	0	9	-2.164e-5	9	736.901	2	NC	1
415	18	max	.024	3	.037	3	.046	4	2.163e-3	4	NC	4	NC	1
416		min	-.027	2	-.051	2	0	9	-1.111e-5	9	1428.653	2	NC	1
417	19	max	.024	3	.062	3	.047	4	6.545e-6	5	NC	1	NC	1
418		min	-.027	2	-.085	2	0	9	-2.06e-6	3	NC	1	NC	1
419	M9	1	max	.009	.024	.005	.005	5	7.243e-3	3	NC	1	NC	1
420		min	-.009	2	-.021	2	0	9	-4.89e-3	2	NC	1	NC	1
421	2	max	.009	3	.014	.005	.005	4	3.56e-3	3	NC	4	NC	1
422		min	-.009	2	-.012	2	0	10	-2.419e-3	2	4649.132	3	NC	1
423	3	max	.009	3	.005	.005	.005	4	8.53e-5	1	NC	4	NC	1
424		min	-.009	2	-.004	2	0	10	-5.422e-5	3	2408.046	3	NC	1
425	4	max	.009	3	.003	.005	.005	4	6.977e-5	1	NC	4	NC	1
426		min	-.009	2	-.004	3	-.001	3	-5.991e-5	3	1715.423	3	NC	1
427	5	max	.009	3	.01	.007	.007	4	5.424e-5	1	NC	4	NC	1
428		min	-.009	2	-.01	3	-.002	3	-6.559e-5	3	1385.049	3	8571.177	3
429	6	max	.009	3	.015	.008	.008	4	3.87e-5	1	NC	4	NC	1
430		min	-.009	2	-.016	3	-.003	3	-7.127e-5	3	1200.589	3	7455.423	3
431	7	max	.009	3	.019	.01	.01	4	2.317e-5	1	NC	4	NC	1
432		min	-.009	2	-.02	3	-.004	3	-7.695e-5	3	1091.332	3	6812.408	3
433	8	max	.009	3	.022	.012	.012	4	7.632e-6	1	NC	4	NC	1
434		min	-.009	2	-.022	3	-.005	3	-8.264e-5	3	1022.927	2	6322.572	4
435	9	max	.009	3	.025	.015	.015	4	4.024e-7	10	NC	4	NC	1
436		min	-.009	2	-.024	3	-.005	3	-8.832e-5	3	970.61	2	4678.866	4
437	10	max	.009	3	.025	.018	.018	5	2.208e-6	10	NC	4	NC	1
438		min	-.009	2	-.024	3	-.005	3	-9.4e-5	3	945.47	2	3642.572	4
439	11	max	.009	3	.025	.021	.021	5	4.256e-6	5	NC	4	NC	1
440		min	-.009	2	-.024	3	-.005	3	-9.968e-5	3	944.632	2	2945.154	4
441	12	max	.008	3	.023	.024	.024	5	9.708e-6	5	NC	4	NC	1



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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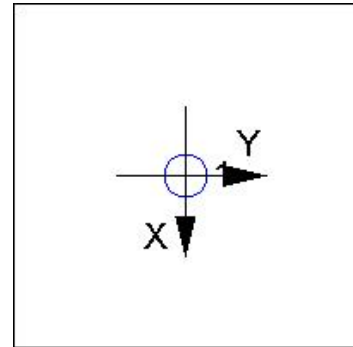
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

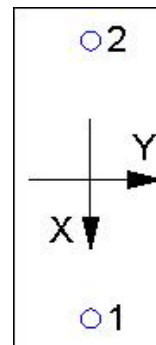
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.