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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

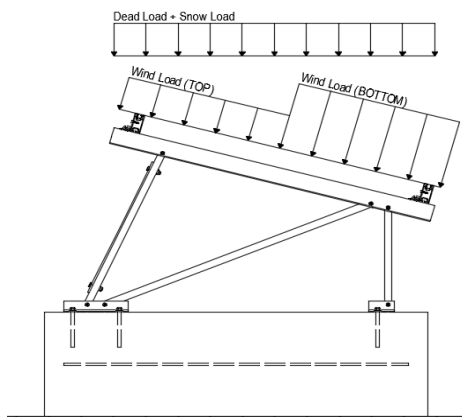
### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	160 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 40.19 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

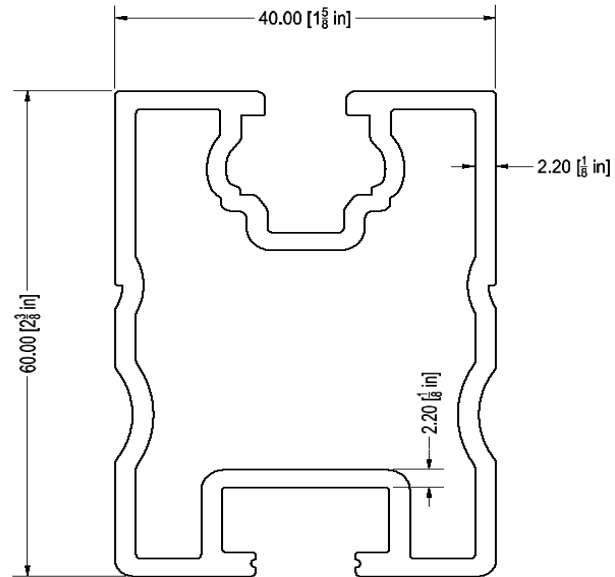
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

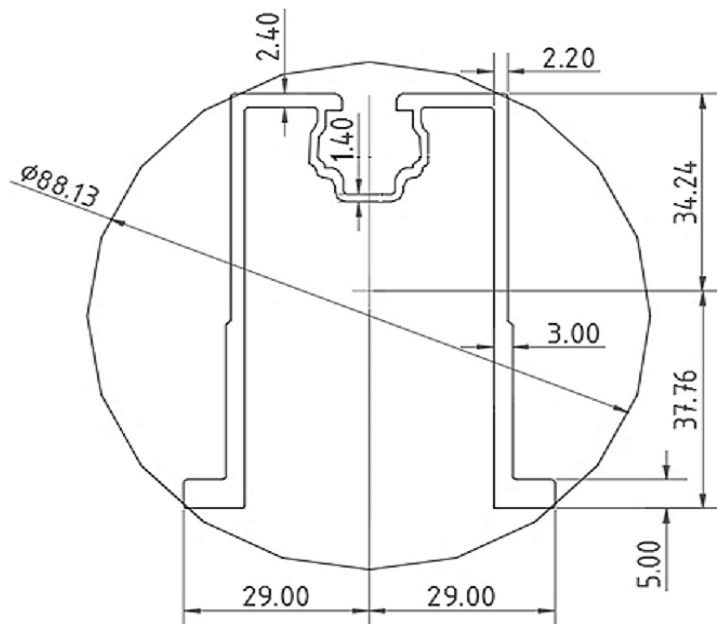
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42 in
$\Phi F_{ty}$ STRONG-AXIS =	29.99 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	-0.330 k-ft
$M_z$ =	-0.015 k-ft
$M_{y \text{ allowable}}$ =	1.276 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>28%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

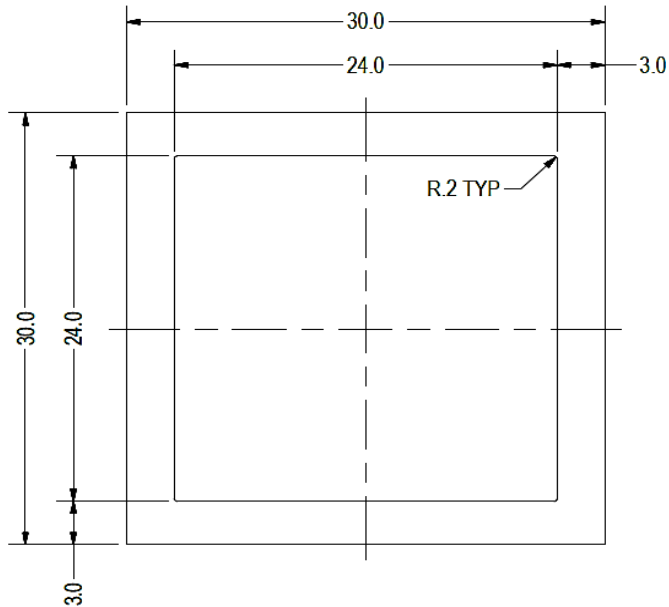
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.73 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	-0.444 k-ft
$M_z$ =	-0.020 k-ft
$P_n$ =	0.148 k
$M_{y \text{ allowable}}$ =	1.459 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>35%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.765 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>6%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.426 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>11%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.613 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<b>11%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.001 k-ft
$P_n$ =	0.112 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>3%</b>



A cross brace kit is required every 63 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

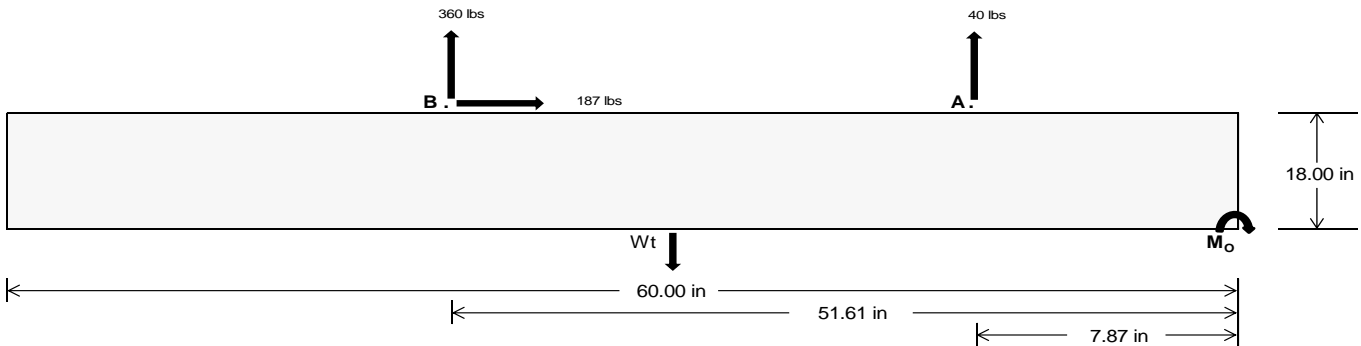
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>180.08</b>	<b>1562.90</b>	k
Compressive Load =	<b>994.88</b>	<b>1012.52</b>	k
Lateral Load =	<b>1.39</b>	<b>810.68</b>	k
Moment (Weak Axis) =	<b>0.00</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 22267.3$  in-lbs  
Resisting Force Required = 742.24 lbs  
S.F. = 1.67  
Weight Required = 1237.07 lbs  
Minimum Width = 20 in  
Weight Provided = 1812.50 lbs

### Sliding

Force = 187.04 lbs  
Friction = 0.4  
Weight Required = 467.61 lbs  
Resisting Weight = 1812.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 187.04 lbs  
Cohesion = 130 psf  
Area = 8.33 ft<sup>2</sup>  
Resisting = 906.25 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
$F_A$	310 lbs	310 lbs	310 lbs	310 lbs	397 lbs	397 lbs	397 lbs	397 lbs	504 lbs	504 lbs	504 lbs	504 lbs	-81 lbs	-81 lbs	-81 lbs	-81 lbs
$F_B$	213 lbs	213 lbs	213 lbs	213 lbs	425 lbs	425 lbs	425 lbs	425 lbs	460 lbs	460 lbs	460 lbs	460 lbs	-720 lbs	-720 lbs	-720 lbs	-720 lbs
$F_V$	21 lbs	21 lbs	21 lbs	21 lbs	331 lbs	331 lbs	331 lbs	331 lbs	263 lbs	263 lbs	263 lbs	263 lbs	-374 lbs	-374 lbs	-374 lbs	-374 lbs
$P_{total}$	2336 lbs	2427 lbs	2518 lbs	2608 lbs	2634 lbs	2725 lbs	2816 lbs	2906 lbs	2777 lbs	2867 lbs	2958 lbs	3048 lbs	286 lbs	341 lbs	395 lbs	450 lbs
$M$	220 lbs-ft	220 lbs-ft	220 lbs-ft	220 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	495 lbs-ft	495 lbs-ft	495 lbs-ft	495 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft
$e$	0.09 ft	0.09 ft	0.09 ft	0.08 ft	0.18 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.05 ft	1.72 ft	1.48 ft	1.30 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	248.7 psf	247.2 psf	245.8 psf	244.6 psf	249.6 psf	248.1 psf	246.7 psf	245.4 psf	261.9 psf	259.8 psf	257.9 psf	256.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	312.0 psf	307.5 psf	303.4 psf	299.7 psf	382.6 psf	374.8 psf	367.6 psf	361.1 psf	404.4 psf	395.5 psf	387.4 psf	380.1 psf	252.9 psf	166.6 psf	141.4 psf	130.8 psf

Maximum Bearing Pressure = 404 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

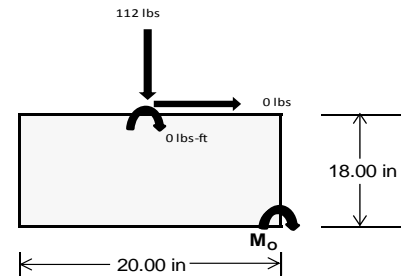
#### Overturning Check

$M_o = 0.0 \text{ ft-lbs}$   
 Resisting Force Required = 0.00 lbs  
 S.F. = 1.67  
 Weight Required = 0.00 lbs  
 Minimum Width = 20 in  
 Weight Provided = 1812.50 lbs

*A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	47 lbs	112 lbs	44 lbs	154 lbs	432 lbs	151 lbs	14 lbs	33 lbs	13 lbs
$F_v$	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2291 lbs	2356 lbs	2288 lbs	2290 lbs	2568 lbs	2287 lbs	670 lbs	689 lbs	669 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
$f_{min}$	274.9 sqft	282.7 sqft	274.6 sqft	274.6 sqft	308.0 sqft	274.4 sqft	80.4 sqft	82.7 sqft	80.3 sqft
$f_{max}$	275.0 psf	282.7 psf	274.6 psf	275.0 psf	308.2 psf	274.6 psf	80.4 psf	82.7 psf	80.3 psf



Maximum Bearing Pressure = 308 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



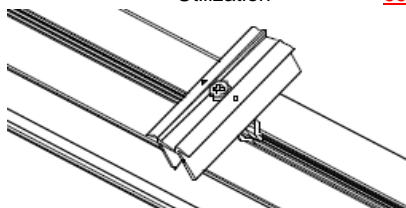
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

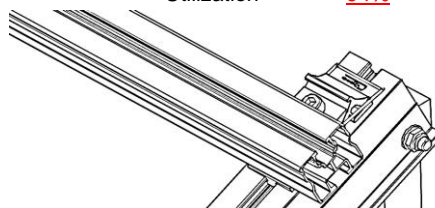
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.834 k
Allowable Uplift =	1.214 k
Utilization =	<u>69%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.044 k
Allowable Uplift =	1.116 k
Utilization =	<u>94%</u>



### 6.2 Bolted Connections

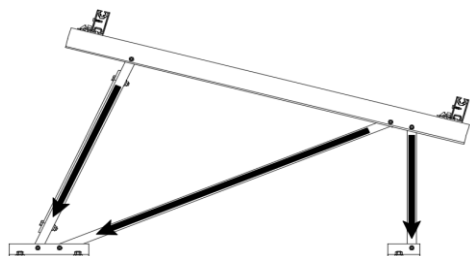
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.765 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

#### Diagonal Strut

Maximum Axial Load =	0.426 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>7%</u>



#### Rear Strut

Maximum Axial Load =	1.033 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

#### Bracing

Maximum Axial Load =	0.112 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	30.83 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.617 in
Max Drift, $\Delta_{MAX}$ =	0.003 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.9$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 23.9$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 30$$

$$Cc = 30$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 250988 \text{ mm}^4$$

$$0.603 \text{ in}^4$$

$$y = 30 \text{ mm}$$

$$S_x = 0.511 \text{ in}^3$$

$$M_{\max} St = 1.276 \text{ k-ft}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20$$

$$Cc = 20$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.5 \text{ ksi}$$

$$I_y = 120291 \text{ mm}^4$$

$$0.289 \text{ in}^4$$

$$x = 20 \text{ mm}$$

$$S_y = 0.367 \text{ in}^3$$

$$M_{\max} Wk = 0.871 \text{ k-ft}$$

### Compression

#### 3.4.9

$$b/t = 7.4$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$
  

$$b/t = 23.9$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi c [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 28.47 \text{ ksi}$$

$$A = 578.06 \text{ mm}^2$$

$$0.90 \text{ in}^2$$

$$P_{\max} = 25.51 \text{ kips}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &21.5728 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y F_{cy} \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.459 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.5514 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.7972 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 11.5927 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 11.59 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 5.82 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-51.748	-51.748	0	0
2	M16	Y	-51.748	-51.748	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-123.3	-123.3	0	0
2	M16	y	-190.554	-190.554	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	246.6	246.6	0	0
2	M16	y	112.091	112.091	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								
8																				
9	ASD 1.0D + 1.0S	Yes	Y		1	1	3	1												
10	ASD 1.0D + 0.6W	Yes	Y		1	1			4	.6										
11	ASD 1.0D + 0.75L + 0.45W + 0....	Yes	Y		1	1	3	.75	4	.45										
12	ASD 0.6D + 0.6W	Yes	Y		2	.6					5	.6								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y		1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y		1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y		1	.362					6	.875								



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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	185.144	2	256.673	2	.005	10	0	10	0	1	0	1
2		min	-218.361	3	-388.827	3	-.168	3	0	3	0	1	0	1
3	N7	max	0	15	265.576	1	.047	10	0	10	0	1	0	1
4		min	-.111	2	-29.897	3	-.406	1	0	1	0	1	0	1
5	N15	max	0	15	765.296	2	.072	9	0	9	0	1	0	1
6		min	-1.066	2	-138.525	3	-.631	3	0	3	0	1	0	1
7	N16	max	559.482	2	778.86	2	0	11	0	9	0	1	0	1
8		min	-623.603	3	-1202.232	3	-83.474	3	0	3	0	1	0	1
9	N23	max	0	15	265.88	1	.422	3	0	1	0	1	0	1
10		min	-.111	2	-29.293	3	-.047	10	0	10	0	1	0	1
11	N24	max	185.145	2	258.87	2	84.187	3	0	9	0	1	0	1
12		min	-218.982	3	-388.316	3	-.006	10	0	3	0	1	0	1
13	Totals:	max	928.483	2	2550.499	2	0	10						
14		min	-1061.126	3	-2177.089	3	0	3						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	198.83	2	.645	4	.068	1	0	10	0	10	0	1
2			min	-348.825	3	.152	15	-.093	3	0	3	0	1	0	1
3		2	max	198.946	2	.599	4	.068	1	0	10	0	15	0	15
4			min	-348.738	3	.141	15	-.093	3	0	3	0	3	0	4
5		3	max	199.063	2	.554	4	.068	1	0	10	0	9	0	15
6			min	-348.65	3	.13	15	-.093	3	0	3	0	3	0	4
7		4	max	199.179	2	.508	4	.068	1	0	10	0	9	0	15
8			min	-348.563	3	.12	15	-.093	3	0	3	0	3	0	4
9		5	max	199.295	2	.462	4	.068	1	0	10	0	9	0	15
10			min	-348.476	3	.109	15	-.093	3	0	3	0	3	0	4
11		6	max	199.412	2	.417	4	.068	1	0	10	0	9	0	15
12			min	-348.389	3	.098	15	-.093	3	0	3	0	3	0	4
13		7	max	199.528	2	.371	4	.068	1	0	10	0	9	0	15
14			min	-348.301	3	.087	15	-.093	3	0	3	0	3	0	4
15		8	max	199.645	2	.325	4	.068	1	0	10	0	9	0	15
16			min	-348.214	3	.077	15	-.093	3	0	3	0	3	0	4
17		9	max	199.761	2	.28	4	.068	1	0	10	0	9	0	15
18			min	-348.127	3	.066	15	-.093	3	0	3	0	3	0	4
19		10	max	199.877	2	.234	4	.068	1	0	10	0	9	0	15
20			min	-348.039	3	.055	15	-.093	3	0	3	0	3	0	4
21		11	max	199.994	2	.188	4	.068	1	0	10	0	9	0	15
22			min	-347.952	3	.045	15	-.093	3	0	3	0	3	0	4
23		12	max	200.11	2	.143	4	.068	1	0	10	0	9	0	15
24			min	-347.865	3	.034	15	-.093	3	0	3	0	3	0	4
25		13	max	200.227	2	.105	2	.068	1	0	10	0	9	0	15
26			min	-347.777	3	.018	12	-.093	3	0	3	0	3	0	4
27		14	max	200.343	2	.07	2	.068	1	0	10	0	9	0	15
28			min	-347.69	3	-.002	3	-.093	3	0	3	0	3	0	4
29		15	max	200.459	2	.034	2	.068	1	0	10	0	9	0	15
30			min	-347.603	3	-.029	3	-.093	3	0	3	0	3	0	4
31		16	max	200.576	2	-.002	2	.068	1	0	10	0	9	0	15
32			min	-347.515	3	-.056	3	-.093	3	0	3	0	3	0	4
33		17	max	200.692	2	-.02	15	.068	1	0	10	0	9	0	15
34			min	-347.428	3	-.086	4	-.093	3	0	3	0	3	0	4
35		18	max	200.809	2	-.031	15	.068	1	0	10	0	9	0	15
36			min	-347.341	3	-.131	4	-.093	3	0	3	0	3	0	4
37		19	max	200.925	2	-.041	15	.068	1	0	10	0	9	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-347.254	3	-.177	4	-.093	3	0	3	0	3	0	4
39	M3	1	max	142.025	2	1.78	4	.01	10	0	10	0	1	4
40		min	-129.741	3	.419	15	-.103	1	0	1	0	10	0	15
41		2	max	141.957	2	1.602	4	.01	10	0	10	0	1	2
42		min	-129.793	3	.377	15	-.103	1	0	1	0	10	0	12
43		3	max	141.888	2	1.425	4	.01	10	0	10	0	1	2
44		min	-129.844	3	.335	15	-.103	1	0	1	0	10	0	3
45		4	max	141.82	2	1.248	4	.01	10	0	10	0	1	15
46		min	-129.896	3	.294	15	-.103	1	0	1	0	10	0	4
47		5	max	141.751	2	1.071	4	.01	10	0	10	0	1	15
48		min	-129.947	3	.252	15	-.103	1	0	1	0	10	0	4
49		6	max	141.682	2	.894	4	.01	10	0	10	0	1	15
50		min	-129.999	3	.21	15	-.103	1	0	1	0	10	0	4
51		7	max	141.614	2	.716	4	.01	10	0	10	0	1	15
52		min	-130.05	3	.169	15	-.103	1	0	1	0	10	0	4
53		8	max	141.545	2	.539	4	.01	10	0	10	0	1	15
54		min	-130.101	3	.127	15	-.103	1	0	1	0	10	-.001	4
55		9	max	141.477	2	.362	4	.01	10	0	10	0	1	15
56		min	-130.153	3	.085	15	-.103	1	0	1	0	10	-.001	4
57		10	max	141.408	2	.185	4	.01	10	0	10	0	1	15
58		min	-130.204	3	.044	15	-.103	1	0	1	0	10	-.001	4
59		11	max	141.339	2	.029	2	.01	10	0	10	0	1	15
60		min	-130.256	3	-.021	3	-.103	1	0	1	0	10	-.001	4
61		12	max	141.271	2	-.04	15	.01	10	0	10	0	1	15
62		min	-130.307	3	-.17	4	-.103	1	0	1	0	10	-.001	4
63		13	max	141.202	2	-.081	15	.01	10	0	10	0	1	15
64		min	-130.359	3	-.347	4	-.103	1	0	1	0	10	-.001	4
65		14	max	141.133	2	-.123	15	.01	10	0	10	0	9	15
66		min	-130.41	3	-.524	4	-.103	1	0	1	0	10	-.001	4
67		15	max	141.065	2	-.165	15	.01	10	0	10	0	9	15
68		min	-130.462	3	-.701	4	-.103	1	0	1	0	10	0	4
69		16	max	140.996	2	-.206	15	.01	10	0	10	0	10	15
70		min	-130.513	3	-.878	4	-.103	1	0	1	0	1	0	4
71		17	max	140.928	2	-.248	15	.01	10	0	10	0	10	15
72		min	-130.565	3	-1.056	4	-.103	1	0	1	0	1	0	4
73		18	max	140.859	2	-.29	15	.01	10	0	10	0	10	15
74		min	-130.616	3	-1.233	4	-.103	1	0	1	0	1	0	4
75		19	max	140.79	2	-.331	15	.01	10	0	10	0	10	1
76		min	-130.667	3	-1.41	4	-.103	1	0	1	0	1	0	1
77	M4	1	max	264.412	1	0	1	.048	10	0	1	0	3	1
78		min	-30.771	3	0	1	-.424	1	0	1	0	2	0	1
79		2	max	264.476	1	0	1	.048	10	0	1	0	10	1
80		min	-30.722	3	0	1	-.424	1	0	1	0	1	0	1
81		3	max	264.541	1	0	1	.048	10	0	1	0	10	1
82		min	-30.674	3	0	1	-.424	1	0	1	0	1	0	1
83		4	max	264.606	1	0	1	.048	10	0	1	0	10	1
84		min	-30.625	3	0	1	-.424	1	0	1	0	1	0	1
85		5	max	264.67	1	0	1	.048	10	0	1	0	10	1
86		min	-30.577	3	0	1	-.424	1	0	1	0	1	0	1
87		6	max	264.735	1	0	1	.048	10	0	1	0	10	1
88		min	-30.528	3	0	1	-.424	1	0	1	0	1	0	1
89		7	max	264.8	1	0	1	.048	10	0	1	0	10	1
90		min	-30.48	3	0	1	-.424	1	0	1	0	1	0	1
91		8	max	264.865	1	0	1	.048	10	0	1	0	10	1
92		min	-30.431	3	0	1	-.424	1	0	1	0	1	0	1
93		9	max	264.929	1	0	1	.048	10	0	1	0	10	1
94		min	-30.383	3	0	1	-.424	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	264.994	1	0	1	.048	10	0	1	0	10	0	1
96		min	-30.334	3	0	1	-.424	1	0	1	0	1	0	1
97	11	max	265.059	1	0	1	.048	10	0	1	0	10	0	1
98		min	-30.285	3	0	1	-.424	1	0	1	0	1	0	1
99	12	max	265.123	1	0	1	.048	10	0	1	0	10	0	1
100		min	-30.237	3	0	1	-.424	1	0	1	0	1	0	1
101	13	max	265.188	1	0	1	.048	10	0	1	0	10	0	1
102		min	-30.188	3	0	1	-.424	1	0	1	0	1	0	1
103	14	max	265.253	1	0	1	.048	10	0	1	0	10	0	1
104		min	-30.14	3	0	1	-.424	1	0	1	0	1	0	1
105	15	max	265.317	1	0	1	.048	10	0	1	0	10	0	1
106		min	-30.091	3	0	1	-.424	1	0	1	0	1	0	1
107	16	max	265.382	1	0	1	.048	10	0	1	0	10	0	1
108		min	-30.043	3	0	1	-.424	1	0	1	0	1	0	1
109	17	max	265.447	1	0	1	.048	10	0	1	0	10	0	1
110		min	-29.994	3	0	1	-.424	1	0	1	0	1	0	1
111	18	max	265.512	1	0	1	.048	10	0	1	0	10	0	1
112		min	-29.946	3	0	1	-.424	1	0	1	0	1	0	1
113	19	max	265.576	1	0	1	.048	10	0	1	0	10	0	1
114		min	-29.897	3	0	1	-.424	1	0	1	0	1	0	1
115	M6	1	max	611.16	2	.643	.013	9	0	3	0	3	0	1
116		min	-1033.428	3	.151	15	-.296	3	0	1	0	1	0	1
117	2	max	611.277	2	.597	4	.013	9	0	3	0	3	0	15
118		min	-1033.34	3	.141	15	-.296	3	0	1	0	1	0	4
119	3	max	611.393	2	.552	4	.013	9	0	3	0	3	0	15
120		min	-1033.253	3	.13	15	-.296	3	0	1	0	1	0	4
121	4	max	611.509	2	.506	4	.013	9	0	3	0	3	0	15
122		min	-1033.166	3	.119	15	-.296	3	0	1	0	1	0	4
123	5	max	611.626	2	.46	4	.013	9	0	3	0	3	0	15
124		min	-1033.078	3	.108	15	-.296	3	0	1	0	1	0	4
125	6	max	611.742	2	.415	4	.013	9	0	3	0	9	0	15
126		min	-1032.991	3	.098	15	-.296	3	0	1	0	1	0	4
127	7	max	611.859	2	.373	2	.013	9	0	3	0	9	0	15
128		min	-1032.904	3	.087	15	-.296	3	0	1	0	3	0	4
129	8	max	611.975	2	.338	2	.013	9	0	3	0	9	0	15
130		min	-1032.817	3	.071	12	-.296	3	0	1	0	3	0	4
131	9	max	612.091	2	.302	2	.013	9	0	3	0	9	0	15
132		min	-1032.729	3	.053	12	-.296	3	0	1	0	3	0	4
133	10	max	612.208	2	.267	2	.013	9	0	3	0	9	0	15
134		min	-1032.642	3	.035	12	-.296	3	0	1	0	3	0	4
135	11	max	612.324	2	.231	2	.013	9	0	3	0	9	0	15
136		min	-1032.555	3	.017	3	-.296	3	0	1	0	3	0	4
137	12	max	612.441	2	.195	2	.013	9	0	3	0	9	0	15
138		min	-1032.467	3	-.01	3	-.296	3	0	1	0	3	0	4
139	13	max	612.557	2	.16	2	.013	9	0	3	0	9	0	15
140		min	-1032.38	3	-.037	3	-.296	3	0	1	0	3	0	2
141	14	max	612.673	2	.124	2	.013	9	0	3	0	9	0	12
142		min	-1032.293	3	-.063	3	-.296	3	0	1	0	3	0	2
143	15	max	612.79	2	.089	2	.013	9	0	3	0	9	0	12
144		min	-1032.205	3	-.09	3	-.296	3	0	1	0	3	0	2
145	16	max	612.906	2	.053	2	.013	9	0	3	0	9	0	12
146		min	-1032.118	3	-.117	3	-.296	3	0	1	0	3	0	2
147	17	max	613.023	2	.018	2	.013	9	0	3	0	9	0	12
148		min	-1032.031	3	-.143	3	-.296	3	0	1	0	3	0	2
149	18	max	613.139	2	-.018	2	.013	9	0	3	0	9	0	12
150		min	-1031.944	3	-.17	3	-.296	3	0	1	0	3	0	2
151	19	max	613.255	2	-.042	15	.013	9	0	3	0	9	0	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1031.856	3	-.197	3	-.296	3	0	1	0	3	0	2
153	M7	1	max	425.879	2	1.78	4	.026	3	0	9	0	9	2
154		min	-334.694	3	.419	15	-.003	9	0	3	0	3	0	12
155		2	max	425.81	2	1.603	4	.026	3	0	9	0	9	2
156		min	-334.746	3	.377	15	-.003	9	0	3	0	3	0	3
157		3	max	425.741	2	1.426	4	.026	3	0	9	0	9	2
158		min	-334.797	3	.335	15	-.003	9	0	3	0	3	0	3
159		4	max	425.673	2	1.249	4	.026	3	0	9	0	9	2
160		min	-334.849	3	.294	15	-.003	9	0	3	0	3	0	3
161		5	max	425.604	2	1.071	4	.026	3	0	9	0	9	15
162		min	-334.9	3	.252	15	-.003	9	0	3	0	3	0	4
163		6	max	425.536	2	.894	4	.026	3	0	9	0	9	15
164		min	-334.952	3	.21	15	-.003	9	0	3	0	3	0	4
165		7	max	425.467	2	.717	4	.026	3	0	9	0	9	15
166		min	-335.003	3	.169	15	-.003	9	0	3	0	3	0	4
167		8	max	425.398	2	.54	4	.026	3	0	9	0	9	15
168		min	-335.055	3	.127	15	-.003	9	0	3	0	3	-.001	4
169		9	max	425.33	2	.363	4	.026	3	0	9	0	9	15
170		min	-335.106	3	.085	15	-.003	9	0	3	0	3	-.001	4
171		10	max	425.261	2	.211	2	.026	3	0	9	0	9	15
172		min	-335.158	3	.032	12	-.003	9	0	3	0	3	-.001	4
173		11	max	425.193	2	.072	2	.026	3	0	9	0	9	15
174		min	-335.209	3	-.063	3	-.003	9	0	3	0	3	-.001	4
175		12	max	425.124	2	-.039	15	.026	3	0	9	0	9	15
176		min	-335.26	3	-.169	4	-.003	9	0	3	0	3	-.001	4
177		13	max	425.055	2	-.081	15	.026	3	0	9	0	9	15
178		min	-335.312	3	-.346	4	-.003	9	0	3	0	3	-.001	4
179		14	max	424.987	2	-.123	15	.026	3	0	9	0	9	15
180		min	-335.363	3	-.523	4	-.003	9	0	3	0	3	-.001	4
181		15	max	424.918	2	-.164	15	.026	3	0	9	0	9	15
182		min	-335.415	3	-.701	4	-.003	9	0	3	0	3	0	4
183		16	max	424.85	2	-.206	15	.026	3	0	9	0	9	15
184		min	-335.466	3	-.878	4	-.003	9	0	3	0	3	0	4
185		17	max	424.781	2	-.248	15	.026	3	0	9	0	9	15
186		min	-335.518	3	-1.055	4	-.003	9	0	3	0	3	0	4
187		18	max	424.712	2	-.289	15	.026	3	0	9	0	9	15
188		min	-335.569	3	-1.232	4	-.003	9	0	3	0	3	0	4
189		19	max	424.644	2	-.331	15	.026	3	0	9	0	9	1
190		min	-335.621	3	-1.409	4	-.003	9	0	3	0	3	0	1
191	M8	1	max	764.131	2	0	1	.076	9	0	1	0	1	1
192		min	-139.399	3	0	1	-.617	3	0	1	0	3	0	1
193		2	max	764.196	2	0	1	.076	9	0	1	0	9	1
194		min	-139.35	3	0	1	-.617	3	0	1	0	3	0	1
195		3	max	764.261	2	0	1	.076	9	0	1	0	9	1
196		min	-139.302	3	0	1	-.617	3	0	1	0	3	0	1
197		4	max	764.325	2	0	1	.076	9	0	1	0	9	1
198		min	-139.253	3	0	1	-.617	3	0	1	0	3	0	1
199		5	max	764.39	2	0	1	.076	9	0	1	0	9	1
200		min	-139.205	3	0	1	-.617	3	0	1	0	3	0	1
201		6	max	764.455	2	0	1	.076	9	0	1	0	9	1
202		min	-139.156	3	0	1	-.617	3	0	1	0	3	0	1
203		7	max	764.519	2	0	1	.076	9	0	1	0	9	1
204		min	-139.108	3	0	1	-.617	3	0	1	0	3	0	1
205		8	max	764.584	2	0	1	.076	9	0	1	0	9	1
206		min	-139.059	3	0	1	-.617	3	0	1	0	3	0	1
207		9	max	764.649	2	0	1	.076	9	0	1	0	9	1
208		min	-139.011	3	0	1	-.617	3	0	1	0	3	0	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	764.714	2	0	1	.076	9	0	1	0	9	0	1
210			min	-138.962	3	0	1	-.617	3	0	1	0	3	0	1
211		11	max	764.778	2	0	1	.076	9	0	1	0	9	0	1
212			min	-138.914	3	0	1	-.617	3	0	1	0	3	0	1
213		12	max	764.843	2	0	1	.076	9	0	1	0	9	0	1
214			min	-138.865	3	0	1	-.617	3	0	1	0	3	0	1
215		13	max	764.908	2	0	1	.076	9	0	1	0	9	0	1
216			min	-138.816	3	0	1	-.617	3	0	1	0	3	0	1
217		14	max	764.972	2	0	1	.076	9	0	1	0	9	0	1
218			min	-138.768	3	0	1	-.617	3	0	1	0	3	0	1
219		15	max	765.037	2	0	1	.076	9	0	1	0	9	0	1
220			min	-138.719	3	0	1	-.617	3	0	1	0	3	0	1
221		16	max	765.102	2	0	1	.076	9	0	1	0	9	0	1
222			min	-138.671	3	0	1	-.617	3	0	1	0	3	0	1
223		17	max	765.167	2	0	1	.076	9	0	1	0	9	0	1
224			min	-138.622	3	0	1	-.617	3	0	1	0	3	0	1
225		18	max	765.231	2	0	1	.076	9	0	1	0	9	0	1
226			min	-138.574	3	0	1	-.617	3	0	1	0	3	0	1
227		19	max	765.296	2	0	1	.076	9	0	1	0	9	0	1
228			min	-138.525	3	0	1	-.617	3	0	1	0	3	0	1
229	M10	1	max	199.996	2	.645	4	.005	10	0	1	0	9	0	1
230			min	-275.945	3	.152	15	-.069	1	0	3	0	3	0	1
231		2	max	200.113	2	.599	4	.005	10	0	1	0	9	0	15
232			min	-275.857	3	.141	15	-.069	1	0	3	0	3	0	4
233		3	max	200.229	2	.554	4	.005	10	0	1	0	9	0	15
234			min	-275.77	3	.13	15	-.069	1	0	3	0	3	0	4
235		4	max	200.346	2	.508	4	.005	10	0	1	0	9	0	15
236			min	-275.683	3	.12	15	-.069	1	0	3	0	3	0	4
237		5	max	200.462	2	.462	4	.005	10	0	1	0	9	0	15
238			min	-275.596	3	.109	15	-.069	1	0	3	0	3	0	4
239		6	max	200.578	2	.417	4	.005	10	0	1	0	9	0	15
240			min	-275.508	3	.098	15	-.069	1	0	3	0	3	0	4
241		7	max	200.695	2	.371	4	.005	10	0	1	0	10	0	15
242			min	-275.421	3	.087	15	-.069	1	0	3	0	3	0	4
243		8	max	200.811	2	.325	4	.005	10	0	1	0	10	0	15
244			min	-275.334	3	.077	15	-.069	1	0	3	0	3	0	4
245		9	max	200.928	2	.28	4	.005	10	0	1	0	10	0	15
246			min	-275.246	3	.066	15	-.069	1	0	3	0	3	0	4
247		10	max	201.044	2	.234	4	.005	10	0	1	0	10	0	15
248			min	-275.159	3	.055	15	-.069	1	0	3	0	3	0	4
249		11	max	201.16	2	.188	4	.005	10	0	1	0	10	0	15
250			min	-275.072	3	.045	15	-.069	1	0	3	0	3	0	4
251		12	max	201.277	2	.143	4	.005	10	0	1	0	10	0	15
252			min	-274.984	3	.034	15	-.069	1	0	3	0	3	0	4
253		13	max	201.393	2	.105	2	.005	10	0	1	0	10	0	15
254			min	-274.897	3	.023	15	-.069	1	0	3	0	3	0	4
255		14	max	201.51	2	.07	2	.005	10	0	1	0	10	0	15
256			min	-274.81	3	.008	12	-.069	1	0	3	0	3	0	4
257		15	max	201.626	2	.034	2	.005	10	0	1	0	10	0	15
258			min	-274.723	3	-.017	3	-.069	1	0	3	0	3	0	4
259		16	max	201.742	2	-.002	2	.005	10	0	1	0	10	0	15
260			min	-274.635	3	-.043	3	-.069	1	0	3	0	3	0	4
261		17	max	201.859	2	-.02	15	.005	10	0	1	0	10	0	15
262			min	-274.548	3	-.086	4	-.069	1	0	3	0	3	0	4
263		18	max	201.975	2	-.031	15	.005	10	0	1	0	10	0	15
264			min	-274.461	3	-.131	4	-.069	1	0	3	0	3	0	4
265		19	max	202.092	2	-.041	15	.005	10	0	1	0	10	0	15



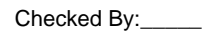
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-274.373	3	-.177	4	-.069	1	0	3	0	3	0	4
267	M11	1	max	141.645	2	1.78	4	.103	1	0	3	0	3	0	4
268			min	-130.577	3	.419	15	-.047	3	0	10	0	1	0	15
269		2	max	141.577	2	1.602	4	.103	1	0	3	0	3	0	2
270			min	-130.629	3	.377	15	-.047	3	0	10	0	1	0	12
271		3	max	141.508	2	1.425	4	.103	1	0	3	0	3	0	2
272			min	-130.68	3	.335	15	-.047	3	0	10	0	1	0	3
273		4	max	141.439	2	1.248	4	.103	1	0	3	0	3	0	15
274			min	-130.732	3	.294	15	-.047	3	0	10	0	1	0	4
275		5	max	141.371	2	1.071	4	.103	1	0	3	0	3	0	15
276			min	-130.783	3	.252	15	-.047	3	0	10	0	1	0	4
277		6	max	141.302	2	.894	4	.103	1	0	3	0	3	0	15
278			min	-130.835	3	.21	15	-.047	3	0	10	0	1	0	4
279		7	max	141.234	2	.716	4	.103	1	0	3	0	3	0	15
280			min	-130.886	3	.169	15	-.047	3	0	10	0	1	0	4
281		8	max	141.165	2	.539	4	.103	1	0	3	0	3	0	15
282			min	-130.937	3	.127	15	-.047	3	0	10	0	1	-.001	4
283		9	max	141.096	2	.362	4	.103	1	0	3	0	3	0	15
284			min	-130.989	3	.085	15	-.047	3	0	10	0	1	-.001	4
285		10	max	141.028	2	.185	4	.103	1	0	3	0	3	0	15
286			min	-131.04	3	.044	15	-.047	3	0	10	0	1	-.001	4
287		11	max	140.959	2	.029	2	.103	1	0	3	0	3	0	15
288			min	-131.092	3	-.026	3	-.047	3	0	10	0	1	-.001	4
289		12	max	140.891	2	-.04	15	.103	1	0	3	0	3	0	15
290			min	-131.143	3	-.17	4	-.047	3	0	10	0	1	-.001	4
291		13	max	140.822	2	-.081	15	.103	1	0	3	0	3	0	15
292			min	-131.195	3	-.347	4	-.047	3	0	10	0	1	-.001	4
293		14	max	140.753	2	-.123	15	.103	1	0	3	0	3	0	15
294			min	-131.246	3	-.524	4	-.047	3	0	10	0	1	-.001	4
295		15	max	140.685	2	-.165	15	.103	1	0	3	0	3	0	15
296			min	-131.298	3	-.701	4	-.047	3	0	10	0	1	0	4
297		16	max	140.616	2	-.206	15	.103	1	0	3	0	3	0	15
298			min	-131.349	3	-.878	4	-.047	3	0	10	0	10	0	4
299		17	max	140.548	2	-.248	15	.103	1	0	3	0	3	0	15
300			min	-131.401	3	-1.056	4	-.047	3	0	10	0	10	0	4
301		18	max	140.479	2	-.29	15	.103	1	0	3	0	3	0	15
302			min	-131.452	3	-1.233	4	-.047	3	0	10	0	10	0	4
303		19	max	140.41	2	-.331	15	.103	1	0	3	0	3	0	1
304			min	-131.503	3	-1.41	4	-.047	3	0	10	0	10	0	1
305	M12	1	max	264.715	1	0	1	.425	1	0	1	0	2	0	1
306			min	-30.166	3	0	1	-.048	10	0	1	0	3	0	1
307		2	max	264.78	1	0	1	.425	1	0	1	0	1	0	1
308			min	-30.118	3	0	1	-.048	10	0	1	0	10	0	1
309		3	max	264.844	1	0	1	.425	1	0	1	0	1	0	1
310			min	-30.069	3	0	1	-.048	10	0	1	0	10	0	1
311		4	max	264.909	1	0	1	.425	1	0	1	0	1	0	1
312			min	-30.02	3	0	1	-.048	10	0	1	0	10	0	1
313		5	max	264.974	1	0	1	.425	1	0	1	0	1	0	1
314			min	-29.972	3	0	1	-.048	10	0	1	0	10	0	1
315		6	max	265.038	1	0	1	.425	1	0	1	0	1	0	1
316			min	-29.923	3	0	1	-.048	10	0	1	0	10	0	1
317		7	max	265.103	1	0	1	.425	1	0	1	0	1	0	1
318			min	-29.875	3	0	1	-.048	10	0	1	0	10	0	1
319		8	max	265.168	1	0	1	.425	1	0	1	0	1	0	1
320			min	-29.826	3	0	1	-.048	10	0	1	0	10	0	1
321		9	max	265.233	1	0	1	.425	1	0	1	0	1	0	1
322			min	-29.778	3	0	1	-.048	10	0	1	0	10	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-60.679	1	-149.901	3	-11.967	1	0	2	-.023	1	0	3
381	M5	1	max	157.062	1	1035.758	3	0	1	0	.012	3	0	3
382		min	-6.635	3	-668.111	2	-75.797	3	0	3	0	11	0	2
383		2	max	157.18	1	1035.568	3	0	1	0	0	9	.144	2
384		min	-6.546	3	-668.364	2	-75.797	3	0	3	-.005	3	-.224	3
385		3	max	147.742	3	4.994	9	8.021	3	0	0	9	.287	2
386		min	-21.654	10	-63.844	2	-.086	9	0	1	-.02	3	-.444	3
387		4	max	147.831	3	4.783	9	8.021	3	0	0	9	.301	2
388		min	-21.556	10	-64.097	2	-.086	9	0	1	-.019	3	-.43	3
389		5	max	147.919	3	4.572	9	8.021	3	0	0	9	.315	2
390		min	-21.458	10	-64.35	2	-.086	9	0	1	-.017	3	-.417	3
391		6	max	148.008	3	4.361	9	8.021	3	0	0	9	.329	2
392		min	-21.359	10	-64.603	2	-.086	9	0	1	-.015	3	-.404	3
393		7	max	148.096	3	4.15	9	8.021	3	0	0	9	.343	2
394		min	-21.261	10	-64.856	2	-.086	9	0	1	-.013	3	-.39	3
395		8	max	148.185	3	3.939	9	8.021	3	0	0	9	.357	2
396		min	-21.163	10	-65.109	2	-.086	9	0	1	-.012	3	-.377	3
397		9	max	148.273	3	3.728	9	8.021	3	0	0	9	.371	2
398		min	-21.064	10	-65.362	2	-.086	9	0	1	-.01	3	-.364	3
399		10	max	148.362	3	3.517	9	8.021	3	0	0	1	.385	2
400		min	-20.966	10	-65.616	2	-.086	9	0	1	-.008	3	-.35	3
401		11	max	148.45	3	3.307	9	8.021	3	0	0	1	.399	2
402		min	-20.868	10	-65.869	2	-.086	9	0	1	-.006	3	-.337	3
403		12	max	148.539	3	3.096	9	8.021	3	0	0	1	.414	2
404		min	-20.769	10	-66.122	2	-.086	9	0	1	-.005	3	-.323	3
405		13	max	148.627	3	2.885	9	8.021	3	0	0	1	.428	2
406		min	-20.671	10	-66.375	2	-.086	9	0	1	-.003	3	-.31	3
407		14	max	148.716	3	2.674	9	8.021	3	0	0	1	.443	2
408		min	-20.573	10	-66.628	2	-.086	9	0	1	-.001	3	-.296	3
409		15	max	148.804	3	2.463	9	8.021	3	0	0	3	.457	2
410		min	-20.474	10	-66.881	2	-.086	9	0	1	0	9	-.282	3
411		16	max	253.183	2	266.778	2	7.995	3	0	.002	3	.469	2
412		min	-59.825	3	-325.831	3	-.088	9	0	1	0	9	-.266	3
413		17	max	253.301	2	266.525	2	7.995	3	0	.003	3	.411	2
414		min	-59.737	3	-326.021	3	-.088	9	0	1	0	9	-.196	3
415		18	max	-.877	3	955.085	2	7.381	3	0	.005	3	.206	2
416		min	-157.224	1	-453.048	3	-.016	9	0	9	0	9	-.098	3
417		19	max	-.788	3	954.831	2	7.381	3	0	.007	3	0	3
418		min	-157.106	1	-453.238	3	-.016	9	0	9	0	9	0	2
419	M9	1	max	60.701	1	329.399	3	80.194	3	0	.002	10	0	2
420		min	2.297	15	-215.909	2	-1.123	10	0	2	-.023	1	0	3
421		2	max	60.819	1	329.209	3	80.194	3	0	.002	10	.047	2
422		min	2.333	15	-216.162	2	-1.123	10	0	2	-.02	1	-.072	3
423		3	max	59.525	3	3.816	9	11.445	1	0	.015	3	.093	2
424		min	-10.198	10	-19.368	2	-2.866	3	0	10	-.017	1	-.142	3
425		4	max	59.613	3	3.605	9	11.445	1	0	.014	3	.098	2
426		min	-10.1	10	-19.622	2	-2.866	3	0	10	-.015	1	-.138	3
427		5	max	59.702	3	3.394	9	11.445	1	0	.014	3	.102	2
428		min	-10.001	10	-19.875	2	-2.866	3	0	10	-.012	1	-.134	3
429		6	max	59.79	3	3.184	9	11.445	1	0	.013	3	.106	2
430		min	-9.903	10	-20.128	2	-2.866	3	0	10	-.01	1	-.131	3
431		7	max	59.879	3	2.973	9	11.445	1	0	.012	3	.111	2
432		min	-9.805	10	-20.381	2	-2.866	3	0	10	-.008	1	-.127	3
433		8	max	59.967	3	2.762	9	11.445	1	0	.012	3	.115	2
434		min	-9.706	10	-20.634	2	-2.866	3	0	10	-.005	1	-.124	3
435		9	max	60.056	3	2.551	9	11.445	1	0	.011	3	.119	2
436		min	-9.608	10	-20.887	2	-2.866	3	0	10	-.003	1	-.12	3



***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	60.144	3	2.34	9	11.445	1	0	1	.01	3	.124	2
438			min	-9.51	10	-21.14	2	-2.866	3	0	10	0	1	-.116	3
439		11	max	60.233	3	2.129	9	11.445	1	0	1	.01	3	.129	2
440			min	-9.411	10	-21.393	2	-2.866	3	0	10	0	10	-.112	3
441		12	max	60.321	3	1.918	9	11.445	1	0	1	.009	3	.133	2
442			min	-9.313	10	-21.646	2	-2.866	3	0	10	0	10	-.108	3
443		13	max	60.41	3	1.707	9	11.445	1	0	1	.009	3	.138	2
444			min	-9.215	10	-21.899	2	-2.866	3	0	10	0	10	-.104	3
445		14	max	60.498	3	1.496	9	11.445	1	0	1	.01	1	.143	2
446			min	-9.116	10	-22.152	2	-2.866	3	0	10	0	10	-.1	3
447		15	max	60.587	3	1.285	9	11.445	1	0	1	.012	1	.148	2
448			min	-9.018	10	-22.405	2	-2.866	3	0	10	-.001	10	-.096	3
449		16	max	83.177	2	86.711	2	11.539	1	0	10	.015	1	.152	2
450			min	-20.678	3	-121.947	3	-2.904	3	0	3	-.001	10	-.092	3
451		17	max	83.295	2	86.458	2	11.539	1	0	10	.017	1	.133	2
452			min	-20.59	3	-122.136	3	-2.904	3	0	3	-.002	10	-.065	3
453		18	max	-2.332	15	307.312	2	11.967	1	0	2	.02	1	.067	2
454			min	-60.797	1	-149.701	3	-2.486	3	0	3	-.002	10	-.033	3
455		19	max	-2.296	15	307.059	2	11.967	1	0	2	.023	1	0	2
456			min	-60.679	1	-149.89	3	-2.486	3	0	3	-.002	10	0	3
457	M13	1	max	80.188	3	215.849	2	-2.297	15	0	2	.023	1	0	2
458			min	-1.123	10	-329.452	3	-60.698	1	0	3	-.002	10	0	3
459		2	max	80.188	3	154.785	2	-1.708	10	0	2	.015	3	.11	3
460			min	-1.123	10	-235.305	3	-45.228	1	0	3	-.004	2	-.072	2
461		3	max	80.188	3	93.722	2	-.156	10	0	2	.012	3	.183	3
462			min	-1.123	10	-141.158	3	-29.758	1	0	3	-.013	1	-.12	2
463		4	max	80.188	3	32.658	2	1.396	10	0	2	.009	3	.22	3
464			min	-1.123	10	-47.012	3	-14.287	1	0	3	-.021	1	-.145	2
465		5	max	80.188	3	47.135	3	4.603	2	0	2	.006	3	.22	3
466			min	-1.123	10	-28.405	2	-6.945	3	0	3	-.024	1	-.146	2
467		6	max	80.188	3	141.281	3	16.653	1	0	2	.003	3	.183	3
468			min	-1.123	10	-89.469	2	-6.127	3	0	3	-.02	1	-.123	2
469		7	max	80.188	3	235.428	3	32.123	1	0	2	.001	3	.11	3
470			min	-1.123	10	-150.532	2	-5.31	3	0	3	-.011	1	-.076	2
471		8	max	80.188	3	329.575	3	47.594	1	0	2	.007	2	0	15
472			min	-1.123	10	-211.596	2	-4.492	3	0	3	0	3	-.006	2
473		9	max	80.188	3	423.721	3	63.064	1	0	2	.026	1	.088	2
474			min	-1.123	10	-272.659	2	-3.674	3	0	3	-.002	3	-.147	3
475		10	max	80.188	3	-6.615	15	78.534	1	0	2	.054	1	.206	2
476			min	-1.123	10	-517.868	3	2.017	12	0	3	-.015	3	-.33	3
477		11	max	11.503	1	272.659	2	4.622	3	0	3	.026	1	.088	2
478			min	-1.123	10	-423.721	3	-63.064	1	0	2	-.014	3	-.147	3
479		12	max	11.503	1	211.596	2	5.44	3	0	3	.007	2	0	15
480			min	-1.123	10	-329.574	3	-47.593	1	0	2	-.012	3	-.006	2
481		13	max	11.503	1	150.532	2	6.257	3	0	3	.001	10	.11	3
482			min	-1.123	10	-235.428	3	-32.123	1	0	2	-.011	1	-.076	2
483		14	max	11.503	1	89.469	2	7.075	3	0	3	0	15	.183	3
484			min	-1.123	10	-141.281	3	-16.653	1	0	2	-.02	1	-.123	2
485		15	max	11.503	1	28.405	2	7.893	3	0	3	0	15	.22	3
486			min	-1.123	10	-47.135	3	-4.603	2	0	2	-.024	1	-.146	2
487		16	max	11.503	1	47.012	3	14.288	1	0	3	0	12	.22	3
488			min	-1.123	10	-32.658	2	-1.396	10	0	2	-.021	1	-.145	2
489		17	max	11.503	1	141.159	3	29.758	1	0	3	.003	3	.183	3
490			min	-1.123	10	-93.722	2	.156	10	0	2	-.013	1	-.12	2
491		18	max	11.503	1	235.305	3	45.228	1	0	3	.007	3	.11	3
492			min	-1.123	10	-154.785	2	1.708	10	0	2	-.004	2	-.072	2
493		19	max	11.503	1	329.452	3	60.698	1	0	3	.023	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-1.123	10	-215.849	2	2.301	15	0	2	-.002	10	0	3
495	M16	1	max	2.489	3	307.133	2	-2.296	15	0	3	.023	1	0	2
496			min	-11.951	1	-149.914	3	-60.682	1	0	2	-.002	10	0	3
497		2	max	2.489	3	219.915	2	-1.708	10	0	3	.003	9	.05	3
498			min	-11.951	1	-108.077	3	-45.212	1	0	2	-.004	2	-.102	2
499		3	max	2.489	3	132.697	2	-.156	10	0	3	0	3	.084	3
500			min	-11.951	1	-66.239	3	-29.742	1	0	2	-.013	1	-.171	2
501		4	max	2.489	3	45.479	2	1.396	10	0	3	0	15	.102	3
502			min	-11.951	1	-24.401	3	-14.272	1	0	2	-.021	1	-.206	2
503		5	max	2.489	3	17.436	3	4.608	2	0	3	0	15	.103	3
504			min	-11.951	1	-41.739	2	-4.405	3	0	2	-.024	1	-.206	2
505		6	max	2.489	3	59.274	3	16.669	1	0	3	0	15	.088	3
506			min	-11.951	1	-128.956	2	-3.587	3	0	2	-.02	1	-.173	2
507		7	max	2.489	3	101.111	3	32.139	1	0	3	.001	10	.057	3
508			min	-11.951	1	-216.174	2	-2.769	3	0	2	-.011	1	-.106	2
509		8	max	2.489	3	142.949	3	47.61	1	0	3	.007	2	.009	3
510			min	-11.951	1	-303.392	2	-1.952	3	0	2	-.008	3	-.005	2
511		9	max	2.489	3	184.786	3	63.08	1	0	3	.026	1	.13	2
512			min	-11.951	1	-390.61	2	-1.134	3	0	2	-.008	3	-.054	3
513		10	max	1.176	10	-6.612	15	78.55	1	0	15	.054	1	.299	2
514			min	-11.951	1	-477.828	2	-1.105	3	0	2	-.008	3	-.134	3
515		11	max	1.176	10	390.61	2	-.288	3	0	2	.026	1	.13	2
516			min	-11.951	1	-184.786	3	-63.08	1	0	3	-.002	3	-.054	3
517		12	max	1.176	10	303.392	2	.53	3	0	2	.007	2	.009	3
518			min	-11.951	1	-142.949	3	-47.609	1	0	3	-.002	3	-.005	2
519		13	max	1.176	10	216.174	2	1.348	3	0	2	.001	10	.057	3
520			min	-11.951	1	-101.111	3	-32.139	1	0	3	-.011	1	-.106	2
521		14	max	1.176	10	128.956	2	2.165	3	0	2	0	12	.088	3
522			min	-11.951	1	-59.274	3	-16.669	1	0	3	-.02	1	-.173	2
523		15	max	1.176	10	41.739	2	2.983	3	0	2	0	3	.103	3
524			min	-11.951	1	-17.436	3	-4.607	2	0	3	-.024	1	-.206	2
525		16	max	1.176	10	24.402	3	14.272	1	0	2	.001	3	.102	3
526			min	-11.951	1	-45.479	2	-1.396	10	0	3	-.021	1	-.206	2
527		17	max	1.176	10	66.239	3	29.742	1	0	2	.003	3	.084	3
528			min	-11.951	1	-132.697	2	.156	10	0	3	-.013	1	-.171	2
529		18	max	1.176	10	108.077	3	45.212	1	0	2	.005	3	.05	3
530			min	-11.951	1	-219.915	2	1.708	10	0	3	-.004	2	-.102	2
531		19	max	1.176	10	149.914	3	60.683	1	0	2	.023	1	0	2
532			min	-11.951	1	-307.133	2	2.299	15	0	3	-.002	10	0	3
533	M15	1	max	0	1	.731	3	.163	3	0	1	0	1	0	1
534			min	-110.834	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.65	3	.163	3	0	1	0	1	0	1
536			min	-110.899	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.569	3	.163	3	0	1	0	1	0	1
538			min	-110.964	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.487	3	.163	3	0	1	0	1	0	1
540			min	-111.029	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.406	3	.163	3	0	1	0	1	0	1
542			min	-111.094	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.325	3	.163	3	0	1	0	1	0	1
544			min	-111.16	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.244	3	.163	3	0	1	0	3	0	1
546			min	-111.225	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.162	3	.163	3	0	1	0	3	0	1
548			min	-111.29	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.081	3	.163	3	0	1	0	3	0	1
550			min	-111.355	3	0	1	0	1	0	3	0	1	0	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.163	3	0	1	0	3	0	1
552		min	-111.42	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.163	3	0	1	0	3	0	1
554		min	-111.485	3	-.081	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.163	3	0	1	0	3	0	1
556		min	-111.551	3	-.162	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.163	3	0	1	0	3	0	1
558		min	-111.616	3	-.244	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.163	3	0	1	0	3	0	1
560		min	-111.681	3	-.325	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.163	3	0	1	0	3	0	1
562		min	-111.746	3	-.406	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.163	3	0	1	0	3	0	1
564		min	-111.811	3	-.487	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.163	3	0	1	0	3	0	1
566		min	-111.877	3	-.569	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.163	3	0	1	0	3	0	1
568		min	-111.942	3	-.65	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.163	3	0	1	0	3	0	1
570		min	-112.007	3	-.731	3	0	1	0	3	0	1	0	1
571	M16A 1	max	0	1	1.251	4	.015	9	0	3	0	3	0	1
572		min	-110.367	3	0	1	-.067	3	0	9	0	9	0	1
573	2	max	0	1	1.112	4	.015	9	0	3	0	3	0	1
574		min	-110.302	3	0	1	-.067	3	0	9	0	9	0	4
575	3	max	0	1	.973	4	.015	9	0	3	0	3	0	1
576		min	-110.236	3	0	1	-.067	3	0	9	0	9	0	4
577	4	max	0	1	.834	4	.015	9	0	3	0	3	0	1
578		min	-110.171	3	0	1	-.067	3	0	9	0	9	0	4
579	5	max	0	1	.695	4	.015	9	0	3	0	3	0	1
580		min	-110.106	3	0	1	-.067	3	0	9	0	9	-.001	4
581	6	max	0	1	.556	4	.015	9	0	3	0	3	0	1
582		min	-110.041	3	0	1	-.067	3	0	9	0	9	-.001	4
583	7	max	0	1	.417	4	.015	9	0	3	0	3	0	1
584		min	-109.976	3	0	1	-.067	3	0	9	0	9	-.001	4
585	8	max	0	1	.278	4	.015	9	0	3	0	3	0	1
586		min	-109.911	3	0	1	-.067	3	0	9	0	9	-.001	4
587	9	max	0	1	.139	4	.015	9	0	3	0	3	0	1
588		min	-109.845	3	0	1	-.067	3	0	9	0	9	-.001	4
589	10	max	0	1	0	1	.015	9	0	3	0	3	0	1
590		min	-109.78	3	0	1	-.067	3	0	9	0	9	-.001	4
591	11	max	.045	13	0	1	.015	9	0	3	0	3	0	1
592		min	-109.715	3	-.139	4	-.067	3	0	9	0	9	-.001	4
593	12	max	.135	13	0	1	.015	9	0	3	0	3	0	1
594		min	-109.65	3	-.278	4	-.067	3	0	9	0	9	-.001	4
595	13	max	.224	13	0	1	.015	9	0	3	0	1	0	1
596		min	-109.585	3	-.417	4	-.067	3	0	9	0	4	-.001	4
597	14	max	.328	4	0	1	.015	9	0	3	0	9	0	1
598		min	-109.519	3	-.556	4	-.067	3	0	9	0	3	-.001	4
599	15	max	.44	4	0	1	.015	9	0	3	0	9	0	1
600		min	-109.454	3	-.695	4	-.067	3	0	9	0	3	-.001	4
601	16	max	.551	4	0	1	.015	9	0	3	0	9	0	1
602		min	-109.389	3	-.834	4	-.067	3	0	9	0	3	0	4
603	17	max	.663	4	0	1	.015	9	0	3	0	9	0	1
604		min	-109.324	3	-.973	4	-.067	3	0	9	0	3	0	4
605	18	max	.775	4	0	1	.015	9	0	3	0	9	0	1
606		min	-109.259	3	-1.112	4	-.067	3	0	9	0	3	0	4
607	19	max	.886	4	0	1	.015	9	0	3	0	9	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-109.193	3	-1.251	4	-.067	3	0	9	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.008	2	.001	9	1.768e-5	10	NC	3	NC	1	
2			min	-.003	3	-.007	3	-.002	3	-1.888e-4	3	4683.096	2	NC	1	
3			2	max	.002	2	.007	2	.001	9	1.685e-5	10	NC	3	NC	1
4				min	-.003	3	-.007	3	-.002	3	-1.788e-4	3	5106.954	2	NC	1
5			3	max	.002	2	.006	2	.001	9	1.602e-5	10	NC	1	NC	1
6				min	-.003	3	-.007	3	-.002	3	-1.689e-4	3	5610.453	2	NC	1
7			4	max	.001	2	.006	2	.001	9	1.519e-5	10	NC	1	NC	1
8				min	-.003	3	-.007	3	-.002	3	-1.59e-4	3	6212.875	2	NC	1
9			5	max	.001	2	.005	2	.001	9	1.436e-5	10	NC	1	NC	1
10				min	-.002	3	-.006	3	-.001	3	-1.49e-4	3	6939.946	2	NC	1
11			6	max	.001	2	.005	2	0	9	1.353e-5	10	NC	1	NC	1
12				min	-.002	3	-.006	3	-.001	3	-1.391e-4	3	7826.57	2	NC	1
13			7	max	.001	2	.004	2	0	9	1.27e-5	10	NC	1	NC	1
14				min	-.002	3	-.006	3	-.001	3	-1.305e-4	1	8921.027	2	NC	1
15			8	max	.001	2	.004	2	0	9	1.187e-5	10	NC	1	NC	1
16				min	-.002	3	-.005	3	0	3	-1.221e-4	1	NC	1	NC	1
17			9	max	0	2	.003	2	0	9	1.104e-5	10	NC	1	NC	1
18				min	-.002	3	-.005	3	0	3	-1.136e-4	1	NC	1	NC	1
19			10	max	0	2	.003	2	0	9	1.021e-5	10	NC	1	NC	1
20				min	-.002	3	-.005	3	0	3	-1.052e-4	1	NC	1	NC	1
21		11	max	0	2	.002	2	0	9	9.383e-6	10	NC	1	NC	1	
22			min	-.001	3	-.004	3	0	3	-9.678e-5	1	NC	1	NC	1	
23		12	max	0	2	.002	2	0	9	8.554e-6	10	NC	1	NC	1	
24			min	-.001	3	-.004	3	0	3	-8.835e-5	1	NC	1	NC	1	
25		13	max	0	2	.001	2	0	9	7.725e-6	10	NC	1	NC	1	
26			min	-.001	3	-.003	3	0	3	-7.991e-5	1	NC	1	NC	1	
27		14	max	0	2	0	2	0	9	6.896e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-7.148e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	9	6.067e-6	10	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-6.304e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	9	5.237e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-5.461e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	9	4.408e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-4.617e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	9	3.579e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-3.774e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.75e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.93e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.372e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.289e-6	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	1.991e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	9	-1.892e-6	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	2.61e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	9	-2.495e-6	10	NC	1	NC	1
45			4	max	0	3	0	2	0	10	3.229e-5	1	NC	1	NC	1
46				min	0	2	-.002	3	0	9	-3.098e-6	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	3.847e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	9	-3.701e-6	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	4.466e-5	1	NC	1	NC	1
50				min	0	2	-.004	3	0	9	-4.304e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	5.085e-5	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	-4.907e-6	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	5.704e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.51e-6	10	NC	1	NC	1
55		9	max	0	3	0	2	0	1	6.323e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-6.113e-6	10	NC	1	NC	1
57		10	max	0	3	.001	2	0	1	6.942e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-6.716e-6	10	NC	1	NC	1
59		11	max	0	3	.002	2	0	1	7.561e-5	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-7.319e-6	10	NC	1	NC	1
61		12	max	0	3	.002	2	0	1	8.18e-5	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-7.922e-6	10	NC	1	NC	1
63		13	max	0	3	.003	2	0	1	8.799e-5	1	NC	1	NC	1
64			min	-.001	2	-.007	3	0	10	-8.525e-6	10	NC	1	NC	1
65		14	max	.001	3	.004	2	0	1	9.418e-5	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	10	-9.128e-6	10	NC	1	NC	1
67		15	max	.001	3	.004	2	.001	1	1.004e-4	1	NC	1	NC	1
68			min	-.001	2	-.007	3	0	10	-9.732e-6	10	NC	1	NC	1
69		16	max	.001	3	.005	2	.001	1	1.066e-4	1	NC	1	NC	1
70			min	-.001	2	-.007	3	0	10	-1.033e-5	10	8625.434	2	NC	1
71		17	max	.001	3	.006	2	.001	1	1.127e-4	1	NC	1	NC	1
72			min	-.001	2	-.008	3	0	10	-1.094e-5	10	7356.474	2	NC	1
73		18	max	.001	3	.007	2	.002	1	1.189e-4	1	NC	1	NC	1
74			min	-.002	2	-.007	3	0	10	-1.154e-5	10	6377.183	2	NC	1
75		19	max	.001	3	.008	2	.002	1	1.251e-4	1	NC	3	NC	1
76			min	-.002	2	-.007	3	0	10	-1.214e-5	10	5613.214	2	NC	1
77	M4	1	max	.001	1	.009	2	0	10	1.332e-5	10	NC	1	NC	1
78			min	0	3	-.008	3	-.001	1	-1.345e-4	1	NC	1	NC	1
79		2	max	.001	1	.008	2	0	10	1.332e-5	10	NC	1	NC	1
80			min	0	3	-.007	3	-.001	1	-1.345e-4	1	NC	1	NC	1
81		3	max	.001	1	.008	2	0	10	1.332e-5	10	NC	1	NC	1
82			min	0	3	-.007	3	-.001	1	-1.345e-4	1	NC	1	NC	1
83		4	max	.001	1	.007	2	0	10	1.332e-5	10	NC	1	NC	1
84			min	0	3	-.006	3	-.001	1	-1.345e-4	1	NC	1	NC	1
85		5	max	0	1	.007	2	0	10	1.332e-5	10	NC	1	NC	1
86			min	0	3	-.006	3	0	1	-1.345e-4	1	NC	1	NC	1
87		6	max	0	1	.006	2	0	10	1.332e-5	10	NC	1	NC	1
88			min	0	3	-.005	3	0	1	-1.345e-4	1	NC	1	NC	1
89		7	max	0	1	.006	2	0	10	1.332e-5	10	NC	1	NC	1
90			min	0	3	-.005	3	0	1	-1.345e-4	1	NC	1	NC	1
91		8	max	0	1	.005	2	0	10	1.332e-5	10	NC	1	NC	1
92			min	0	3	-.005	3	0	1	-1.345e-4	1	NC	1	NC	1
93		9	max	0	1	.005	2	0	10	1.332e-5	10	NC	1	NC	1
94			min	0	3	-.004	3	0	1	-1.345e-4	1	NC	1	NC	1
95		10	max	0	1	.004	2	0	10	1.332e-5	10	NC	1	NC	1
96			min	0	3	-.004	3	0	1	-1.345e-4	1	NC	1	NC	1
97		11	max	0	1	.004	2	0	10	1.332e-5	10	NC	1	NC	1
98			min	0	3	-.003	3	0	1	-1.345e-4	1	NC	1	NC	1
99		12	max	0	1	.003	2	0	10	1.332e-5	10	NC	1	NC	1
100			min	0	3	-.003	3	0	1	-1.345e-4	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	10	1.332e-5	10	NC	1	NC	1
102			min	0	3	-.003	3	0	1	-1.345e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	10	1.332e-5	10	NC	1	NC	1
104			min	0	3	-.002	3	0	1	-1.345e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	10	1.332e-5	10	NC	1	NC	1
106			min	0	3	-.002	3	0	1	-1.345e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	10	1.332e-5	10	NC	1	NC	1
108			min	0	3	-.001	3	0	1	-1.345e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	10	1.332e-5	10	NC	1	NC	1
110			min	0	3	0	3	0	1	-1.345e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	1.332e-5	10	NC	1	NC	1
112			min	0	3	0	3	0	1	-1.345e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	1.332e-5	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.345e-4	1	NC	1	NC	1
115	M6	1	max	.005	2	.024	2	0	9	4.326e-4	3	NC	3	NC	1
116			min	-.009	3	-.021	3	-.006	3	-9.028e-8	1	1524.508	2	6217.596	3
117		2	max	.005	2	.022	2	0	9	4.204e-4	3	NC	3	NC	1
118			min	-.009	3	-.02	3	-.005	3	-8.525e-8	1	1631.895	2	6618.147	3
119		3	max	.005	2	.021	2	0	9	4.082e-4	3	NC	3	NC	1
120			min	-.008	3	-.019	3	-.005	3	-1.884e-7	9	1755.045	2	7092.351	3
121		4	max	.005	2	.019	2	0	9	3.96e-4	3	NC	3	NC	1
122			min	-.008	3	-.018	3	-.005	3	-9.429e-7	9	1897.14	2	7656.237	3
123		5	max	.004	2	.018	2	0	9	3.838e-4	3	NC	3	NC	1
124			min	-.007	3	-.017	3	-.004	3	-1.697e-6	9	2062.278	2	8331.05	3
125		6	max	.004	2	.016	2	0	9	3.715e-4	3	NC	3	NC	1
126			min	-.007	3	-.016	3	-.004	3	-2.452e-6	9	2255.831	2	9145.371	3
127		7	max	.004	2	.015	2	0	9	3.593e-4	3	NC	3	NC	1
128			min	-.006	3	-.015	3	-.004	3	-3.206e-6	9	2484.973	2	NC	1
129		8	max	.003	2	.013	2	0	9	3.471e-4	3	NC	3	NC	1
130			min	-.006	3	-.014	3	-.003	3	-3.961e-6	9	2759.494	2	NC	1
131		9	max	.003	2	.012	2	0	9	3.349e-4	3	NC	3	NC	1
132			min	-.005	3	-.012	3	-.003	3	-4.715e-6	9	3093.112	2	NC	1
133		10	max	.003	2	.01	2	0	9	3.227e-4	3	NC	3	NC	1
134			min	-.005	3	-.011	3	-.002	3	-5.47e-6	9	3505.652	2	NC	1
135		11	max	.002	2	.009	2	0	9	3.104e-4	3	NC	3	NC	1
136			min	-.004	3	-.01	3	-.002	3	-6.224e-6	9	4026.857	2	NC	1
137		12	max	.002	2	.008	2	0	9	2.982e-4	3	NC	3	NC	1
138			min	-.004	3	-.009	3	-.002	3	-6.978e-6	9	4703.472	2	NC	1
139		13	max	.002	2	.006	2	0	9	2.86e-4	3	NC	3	NC	1
140			min	-.003	3	-.008	3	-.001	3	-7.733e-6	9	5613.41	2	NC	1
141		14	max	.002	2	.005	2	0	9	2.738e-4	3	NC	1	NC	1
142			min	-.003	3	-.006	3	-.001	3	-8.487e-6	9	6896.906	2	NC	1
143		15	max	.001	2	.004	2	0	9	2.616e-4	3	NC	1	NC	1
144			min	-.002	3	-.005	3	0	3	-9.242e-6	9	8834.416	2	NC	1
145		16	max	0	2	.003	2	0	9	2.493e-4	3	NC	1	NC	1
146			min	-.002	3	-.004	3	0	3	-9.996e-6	9	NC	1	NC	1
147		17	max	0	2	.002	2	0	9	2.371e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-1.075e-5	9	NC	1	NC	1
149		18	max	0	2	0	2	0	9	2.249e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-1.151e-5	9	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.127e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.226e-5	9	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	5.675e-6	9	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.848e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	5.064e-6	9	NC	1	NC	1
156			min	0	2	-.002	3	0	9	-7.514e-5	3	NC	1	NC	1
157		3	max	0	3	.002	2	0	3	4.454e-6	9	NC	1	NC	1
158			min	0	2	-.003	3	0	9	-5.181e-5	3	NC	1	NC	1
159		4	max	0	3	.003	2	.001	3	3.843e-6	9	NC	1	NC	1
160			min	0	2	-.005	3	0	9	-2.848e-5	3	NC	1	NC	1
161		5	max	0	3	.004	2	.002	3	3.233e-6	9	NC	1	NC	1
162			min	-.001	2	-.007	3	0	9	-5.142e-6	3	NC	1	NC	1
163		6	max	.001	3	.005	2	.002	3	1.819e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	9	0	1	8505.821	2	NC	1
165		7	max	.001	3	.007	2	.002	3	4.153e-5	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.01	3	0	9	0	10	7053.472	2	NC	1
167		8	max	.001	3	.008	2	.003	3	6.486e-5	3	NC	1	NC	1
168			min	-.002	2	-.011	3	0	9	0	5	5983.524	2	NC	1
169		9	max	.002	3	.009	2	.003	3	8.819e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	9	-4.869e-8	13	5157.774	2	NC	1
171		10	max	.002	3	.01	2	.003	3	1.115e-4	3	NC	3	NC	1
172			min	-.002	2	-.014	3	0	9	-1.19e-7	13	4499.64	2	NC	1
173		11	max	.002	3	.012	2	.003	3	1.349e-4	3	NC	3	NC	1
174			min	-.003	2	-.015	3	0	9	-4.306e-7	9	3963.054	2	NC	1
175		12	max	.002	3	.013	2	.003	3	1.582e-4	3	NC	3	NC	1
176			min	-.003	2	-.016	3	0	9	-1.041e-6	9	3518.394	2	NC	1
177		13	max	.003	3	.015	2	.003	3	1.815e-4	3	NC	3	NC	1
178			min	-.003	2	-.017	3	0	9	-1.652e-6	9	3145.531	2	NC	1
179		14	max	.003	3	.016	2	.003	3	2.049e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	0	9	-2.262e-6	9	2830.137	2	NC	1
181		15	max	.003	3	.018	2	.003	3	2.282e-4	3	NC	3	NC	1
182			min	-.004	2	-.019	3	0	9	-2.873e-6	9	2561.624	2	NC	1
183		16	max	.003	3	.02	2	.003	3	2.515e-4	3	NC	3	NC	1
184			min	-.004	2	-.02	3	0	9	-3.483e-6	9	2331.919	2	NC	1
185		17	max	.003	3	.022	2	.003	3	2.749e-4	3	NC	3	NC	1
186			min	-.004	2	-.02	3	0	9	-4.094e-6	9	2134.73	2	NC	1
187		18	max	.004	3	.023	2	.003	3	2.982e-4	3	NC	3	NC	1
188			min	-.005	2	-.021	3	0	9	-4.704e-6	9	1965.059	2	NC	1
189		19	max	.004	3	.025	2	.003	3	3.215e-4	3	NC	3	NC	1
190			min	-.005	2	-.022	3	0	9	-5.315e-6	9	1818.892	2	NC	1
191	M8	1	max	.004	2	.027	2	0	9	-1.018e-7	10	NC	1	NC	1
192			min	0	3	-.022	3	-.002	3	-2.366e-4	3	NC	1	9912.078	3
193		2	max	.003	2	.025	2	0	9	-1.018e-7	10	NC	1	NC	1
194			min	0	3	-.02	3	-.002	3	-2.366e-4	3	NC	1	NC	1
195		3	max	.003	2	.024	2	0	9	-1.018e-7	10	NC	1	NC	1
196			min	0	3	-.019	3	-.002	3	-2.366e-4	3	NC	1	NC	1
197		4	max	.003	2	.022	2	0	9	-1.018e-7	10	NC	1	NC	1
198			min	0	3	-.018	3	-.001	3	-2.366e-4	3	NC	1	NC	1
199		5	max	.003	2	.021	2	0	9	-1.018e-7	10	NC	1	NC	1
200			min	0	3	-.017	3	-.001	3	-2.366e-4	3	NC	1	NC	1
201		6	max	.003	2	.019	2	0	9	-1.018e-7	10	NC	1	NC	1
202			min	0	3	-.016	3	-.001	3	-2.366e-4	3	NC	1	NC	1
203		7	max	.002	2	.018	2	0	9	-1.018e-7	10	NC	1	NC	1
204			min	0	3	-.014	3	-.001	3	-2.366e-4	3	NC	1	NC	1
205		8	max	.002	2	.016	2	0	9	-1.018e-7	10	NC	1	NC	1
206			min	0	3	-.013	3	0	3	-2.366e-4	3	NC	1	NC	1
207		9	max	.002	2	.015	2	0	9	-1.018e-7	10	NC	1	NC	1
208			min	0	3	-.012	3	0	3	-2.366e-4	3	NC	1	NC	1
209		10	max	.002	2	.013	2	0	9	-1.018e-7	10	NC	1	NC	1
210			min	0	3	-.011	3	0	3	-2.366e-4	3	NC	1	NC	1
211		11	max	.002	2	.012	2	0	9	-1.018e-7	10	NC	1	NC	1
212			min	0	3	-.01	3	0	3	-2.366e-4	3	NC	1	NC	1
213		12	max	.001	2	.01	2	0	9	-1.018e-7	10	NC	1	NC	1
214			min	0	3	-.008	3	0	3	-2.366e-4	3	NC	1	NC	1
215		13	max	.001	2	.009	2	0	9	-1.018e-7	10	NC	1	NC	1
216			min	0	3	-.007	3	0	3	-2.366e-4	3	NC	1	NC	1
217		14	max	.001	2	.007	2	0	9	-1.018e-7	10	NC	1	NC	1
218			min	0	3	-.006	3	0	3	-2.366e-4	3	NC	1	NC	1
219		15	max	0	2	.006	2	0	9	-1.018e-7	10	NC	1	NC	1
220			min	0	3	-.005	3	0	3	-2.366e-4	3	NC	1	NC	1
221		16	max	0	2	.004	2	0	9	-1.018e-7	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-2.366e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	2	.003	2	0	9	-1.018e-7	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-2.366e-4	3	NC	1	NC	1
225		18	max	0	2	.001	2	0	9	-1.018e-7	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-2.366e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.018e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.366e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.008	2	0	10	1.809e-4	1	NC	3	NC	1
230			min	-.002	3	-.007	3	-.001	1	-5.284e-4	3	4688.289	2	NC	1
231		2	max	.002	2	.007	2	0	10	1.725e-4	1	NC	3	NC	1
232			min	-.002	3	-.007	3	-.001	1	-5.112e-4	3	5112.764	2	NC	1
233		3	max	.002	2	.006	2	0	3	1.641e-4	1	NC	1	NC	1
234			min	-.002	3	-.007	3	-.001	1	-4.94e-4	3	5617.024	2	NC	1
235		4	max	.002	2	.006	2	0	3	1.557e-4	1	NC	1	NC	1
236			min	-.002	3	-.007	3	-.001	1	-4.768e-4	3	6220.393	2	NC	1
237		5	max	.001	2	.005	2	0	3	1.473e-4	1	NC	1	NC	1
238			min	-.002	3	-.006	3	-.001	1	-4.596e-4	3	6948.656	2	NC	1
239		6	max	.001	2	.005	2	0	3	1.388e-4	1	NC	1	NC	1
240			min	-.002	3	-.006	3	0	1	-4.425e-4	3	7836.798	2	NC	1
241		7	max	.001	2	.004	2	0	3	1.304e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	0	1	-4.253e-4	3	8933.218	2	NC	1
243		8	max	.001	2	.004	2	0	3	1.22e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	0	1	-4.081e-4	3	NC	1	NC	1
245		9	max	.001	2	.003	2	0	3	1.136e-4	1	NC	1	NC	1
246			min	-.001	3	-.005	3	0	1	-3.909e-4	3	NC	1	NC	1
247		10	max	0	2	.003	2	0	3	1.051e-4	1	NC	1	NC	1
248			min	-.001	3	-.005	3	0	1	-3.737e-4	3	NC	1	NC	1
249		11	max	0	2	.002	2	0	3	9.67e-5	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-3.565e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	8.828e-5	1	NC	1	NC	1
252			min	0	3	-.004	3	0	1	-3.393e-4	3	NC	1	NC	1
253		13	max	0	2	.001	2	0	3	7.985e-5	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-3.221e-4	3	NC	1	NC	1
255		14	max	0	2	0	2	0	3	7.143e-5	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-3.05e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	6.3e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.878e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	5.458e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.706e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	4.616e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.534e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	3.773e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.362e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.931e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.19e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.021e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.372e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.926e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.99e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.643e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-2.607e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	1	3.36e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-3.225e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	1	1.077e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-3.843e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	1	4.352e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-4.46e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	4.965e-6	10	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-5.078e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	5.577e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-5.772e-5	3	NC	1	NC	1
283		9	max	0	3	0	2	0	10	6.19e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-8.055e-5	3	NC	1	NC	1
285		10	max	0	3	.001	2	0	10	6.803e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-1.034e-4	3	NC	1	NC	1
287		11	max	0	3	.002	2	0	10	7.415e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-1.262e-4	3	NC	1	NC	1
289		12	max	0	3	.002	2	0	10	8.028e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-1.49e-4	3	NC	1	NC	1
291		13	max	0	3	.003	2	0	10	8.64e-6	10	NC	1	NC	1
292			min	-.001	2	-.007	3	-.003	3	-1.719e-4	3	NC	1	NC	1
293		14	max	.001	3	.004	2	0	10	9.253e-6	10	NC	1	NC	1
294			min	-.001	2	-.007	3	-.003	3	-1.947e-4	3	NC	1	NC	1
295		15	max	.001	3	.004	2	0	10	9.865e-6	10	NC	1	NC	1
296			min	-.001	2	-.007	3	-.003	3	-2.175e-4	3	NC	1	NC	1
297		16	max	.001	3	.005	2	0	10	1.048e-5	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.003	3	-2.404e-4	3	8635.873	2	NC	1
299		17	max	.001	3	.006	2	0	10	1.109e-5	10	NC	1	NC	1
300			min	-.001	2	-.008	3	-.003	3	-2.632e-4	3	7364.503	2	NC	1
301		18	max	.001	3	.007	2	0	10	1.17e-5	10	NC	1	NC	1
302			min	-.002	2	-.008	3	-.002	3	-2.86e-4	3	6383.544	2	NC	1
303		19	max	.001	3	.008	2	0	10	1.232e-5	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.002	3	-3.088e-4	3	5618.394	2	NC	1
305	M12	1	max	.001	1	.009	2	.001	1	3.28e-4	3	NC	1	NC	1
306			min	0	3	-.008	3	0	10	-1.352e-5	10	NC	1	NC	1
307		2	max	.001	1	.008	2	.001	1	3.28e-4	3	NC	1	NC	1
308			min	0	3	-.007	3	0	10	-1.352e-5	10	NC	1	NC	1
309		3	max	.001	1	.008	2	.001	1	3.28e-4	3	NC	1	NC	1
310			min	0	3	-.007	3	0	10	-1.352e-5	10	NC	1	NC	1
311		4	max	.001	1	.007	2	.001	1	3.28e-4	3	NC	1	NC	1
312			min	0	3	-.006	3	0	10	-1.352e-5	10	NC	1	NC	1
313		5	max	0	1	.007	2	0	1	3.28e-4	3	NC	1	NC	1
314			min	0	3	-.006	3	0	10	-1.352e-5	10	NC	1	NC	1
315		6	max	0	1	.006	2	0	1	3.28e-4	3	NC	1	NC	1
316			min	0	3	-.005	3	0	10	-1.352e-5	10	NC	1	NC	1
317		7	max	0	1	.006	2	0	1	3.28e-4	3	NC	1	NC	1
318			min	0	3	-.005	3	0	10	-1.352e-5	10	NC	1	NC	1
319		8	max	0	1	.005	2	0	1	3.28e-4	3	NC	1	NC	1
320			min	0	3	-.005	3	0	10	-1.352e-5	10	NC	1	NC	1
321		9	max	0	1	.005	2	0	1	3.28e-4	3	NC	1	NC	1
322			min	0	3	-.004	3	0	10	-1.352e-5	10	NC	1	NC	1
323		10	max	0	1	.004	2	0	1	3.28e-4	3	NC	1	NC	1
324			min	0	3	-.004	3	0	10	-1.352e-5	10	NC	1	NC	1
325		11	max	0	1	.004	2	0	1	3.28e-4	3	NC	1	NC	1
326			min	0	3	-.003	3	0	10	-1.352e-5	10	NC	1	NC	1
327		12	max	0	1	.003	2	0	1	3.28e-4	3	NC	1	NC	1
328			min	0	3	-.003	3	0	10	-1.352e-5	10	NC	1	NC	1
329		13	max	0	1	.003	2	0	1	3.28e-4	3	NC	1	NC	1
330			min	0	3	-.003	3	0	10	-1.352e-5	10	NC	1	NC	1
331		14	max	0	1	.002	2	0	1	3.28e-4	3	NC	1	NC	1
332			min	0	3	-.002	3	0	10	-1.352e-5	10	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.28e-4	3	NC	1	NC	1
334			min	0	3	-.002	3	0	10	-1.352e-5	10	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	3.28e-4	3	NC	1	NC	1
336			min	0	3	-.001	3	0	10	-1.352e-5	10	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	3.28e-4	3	NC	1	NC	1
338			min	0	3	0	3	0	10	-1.352e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.28e-4	3	NC	1	NC	1
340			min	0	3	0	3	0	10	-1.352e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.28e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.352e-5	10	NC	1	NC	1
343	M1	1	max	.007	3	.022	3	.003	3	4.721e-3	2	NC	1	NC	1
344			min	-.007	2	-.019	2	0	9	-6.787e-3	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.003	3	2.335e-3	2	NC	4	NC	1
346			min	-.007	2	-.01	2	-.001	9	-3.331e-3	3	5047.71	3	NC	1
347		3	max	.007	3	.004	3	.002	3	6.168e-5	3	NC	4	NC	1
348			min	-.007	2	-.003	2	-.002	9	-7.311e-5	1	2617.759	3	NC	1
349		4	max	.007	3	.004	2	.002	3	6.158e-5	3	NC	4	NC	1
350			min	-.007	2	-.003	3	-.002	9	-6.044e-5	9	1869.633	3	NC	1
351		5	max	.007	3	.01	2	.001	3	6.147e-5	3	NC	4	NC	1
352			min	-.007	2	-.01	3	-.002	1	-4.838e-5	9	1513.75	3	NC	1
353		6	max	.007	3	.015	2	.001	3	6.137e-5	3	NC	4	NC	1
354			min	-.007	2	-.015	3	-.002	1	-3.632e-5	9	1315.927	3	NC	1
355		7	max	.007	3	.019	2	0	3	6.127e-5	3	NC	4	NC	1
356			min	-.007	2	-.018	3	-.001	9	-2.427e-5	9	1195.891	2	NC	1
357		8	max	.007	3	.022	2	0	3	6.117e-5	3	NC	4	NC	1
358			min	-.007	2	-.021	3	-.001	9	-1.221e-5	9	1104.483	2	NC	1
359		9	max	.007	3	.024	2	0	3	6.107e-5	3	NC	4	NC	1
360			min	-.007	2	-.022	3	0	9	-1.124e-6	10	1050.36	2	NC	1
361		10	max	.007	3	.024	2	0	3	6.097e-5	3	NC	4	NC	1
362			min	-.007	2	-.023	3	0	9	-2.623e-6	10	1025.355	2	NC	1
363		11	max	.007	3	.024	2	0	3	6.087e-5	3	NC	4	NC	1
364			min	-.007	2	-.022	3	0	10	-4.122e-6	10	1026.524	2	NC	1
365		12	max	.007	3	.022	2	0	3	6.077e-5	3	NC	4	NC	1
366			min	-.007	2	-.02	3	0	10	-5.622e-6	10	1055.059	2	NC	1
367		13	max	.007	3	.02	2	.001	1	7.517e-5	1	NC	4	NC	1
368			min	-.007	2	-.017	3	0	10	-7.121e-6	10	1116.922	2	NC	1
369		14	max	.007	3	.016	2	.002	1	9.e-5	1	NC	4	NC	1
370			min	-.007	2	-.013	3	0	10	-8.62e-6	10	1225.877	2	NC	1
371		15	max	.007	3	.01	2	.002	1	1.048e-4	1	NC	4	NC	1
372			min	-.007	2	-.009	3	0	10	-1.012e-5	10	1412.127	2	NC	1
373		16	max	.007	3	.004	2	.002	1	1.16e-4	1	NC	4	NC	1
374			min	-.007	2	-.003	3	0	10	-1.124e-5	10	1749.511	2	NC	1
375		17	max	.007	3	.003	3	.001	1	6.502e-5	3	NC	4	NC	1
376			min	-.007	2	-.005	2	0	10	-3.462e-6	10	2473.99	2	NC	1
377		18	max	.007	3	.01	3	0	3	3.259e-3	2	NC	4	NC	1
378			min	-.007	2	-.014	2	0	10	-1.715e-3	3	4791.451	2	NC	1
379		19	max	.007	3	.018	3	0	3	6.575e-3	2	NC	1	NC	1
380			min	-.007	2	-.024	2	0	9	-3.529e-3	3	NC	1	NC	1
381	M5	1	max	.02	3	.068	3	.003	3	9.04e-6	3	NC	1	NC	1
382			min	-.023	2	-.057	2	0	9	0	15	NC	1	NC	1
383		2	max	.02	3	.039	3	.005	3	1.204e-4	3	NC	4	NC	1
384			min	-.023	2	-.032	2	0	9	-7.772e-6	9	1647.339	3	NC	1
385		3	max	.02	3	.012	3	.006	3	2.295e-4	3	NC	5	NC	1
386			min	-.023	2	-.008	2	0	9	-1.546e-5	9	854.9	3	NC	1
387		4	max	.02	3	.013	2	.007	3	2.231e-4	3	NC	5	NC	1
388			min	-.023	2	-.011	3	0	9	-1.457e-5	9	611.561	3	NC	1
389		5	max	.02	3	.031	2	.007	3	2.168e-4	3	NC	5	NC	1
390			min	-.023	2	-.03	3	0	9	-1.369e-5	9	495.987	3	NC	1
391		6	max	.02	3	.046	2	.008	3	2.104e-4	3	NC	5	NC	1
392			min	-.023	2	-.045	3	0	9	-1.28e-5	9	431.912	3	9175.906	3
393		7	max	.02	3	.058	2	.008	3	2.04e-4	3	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.023	2	-.056	3	0	9	-1.191e-5	9	385.947	2	8733.874	3
395		8	max	.02	3	.067	2	.008	3	1.976e-4	3	NC	5	NC	1
396			min	-.023	2	-.063	3	0	9	-1.102e-5	9	356.397	2	8649.932	3
397		9	max	.02	3	.073	2	.007	3	1.913e-4	3	NC	5	NC	1
398			min	-.023	2	-.068	3	0	9	-1.013e-5	9	338.903	2	8858.428	3
399		10	max	.02	3	.076	2	.007	3	1.849e-4	3	NC	5	NC	1
400			min	-.023	2	-.068	3	0	9	-9.245e-6	9	330.821	2	9353.999	3
401		11	max	.02	3	.074	2	.006	3	1.785e-4	3	NC	5	NC	1
402			min	-.023	2	-.066	3	0	9	-8.357e-6	9	331.199	2	NC	1
403		12	max	.02	3	.07	2	.006	3	1.722e-4	3	NC	5	NC	1
404			min	-.023	2	-.06	3	0	9	-7.469e-6	9	340.422	2	NC	1
405		13	max	.019	3	.061	2	.005	3	1.658e-4	3	NC	5	NC	1
406			min	-.023	2	-.052	3	0	9	-6.58e-6	9	360.413	2	NC	1
407		14	max	.019	3	.048	2	.004	3	1.594e-4	3	NC	5	NC	1
408			min	-.023	2	-.041	3	0	9	-5.692e-6	9	395.618	2	NC	1
409		15	max	.019	3	.032	2	.003	3	1.53e-4	3	NC	5	NC	1
410			min	-.023	2	-.026	3	0	9	-4.804e-6	9	455.789	2	NC	1
411		16	max	.019	3	.011	2	.003	3	1.427e-4	3	NC	5	NC	1
412			min	-.023	2	-.01	3	0	9	-4.52e-6	9	564.764	2	NC	1
413		17	max	.019	3	.009	3	.002	3	3.783e-5	3	NC	5	NC	1
414			min	-.023	2	-.014	2	0	9	-1.863e-5	9	798.701	2	NC	1
415		18	max	.019	3	.03	3	.001	3	1.741e-5	3	NC	4	NC	1
416			min	-.023	2	-.044	2	0	9	-9.569e-6	9	1547.126	2	NC	1
417		19	max	.019	3	.052	3	0	3	-3.293e-8	15	NC	1	NC	1
418			min	-.023	2	-.075	2	0	9	-1.422e-6	3	NC	1	NC	1
419	M9	1	max	.007	3	.022	3	.003	3	6.805e-3	3	NC	1	NC	1
420			min	-.007	2	-.019	2	0	9	-4.721e-3	2	NC	1	NC	1
421		2	max	.007	3	.012	3	.001	3	3.361e-3	3	NC	4	NC	1
422			min	-.007	2	-.01	2	0	10	-2.335e-3	2	5050.524	3	NC	1
423		3	max	.007	3	.003	3	.001	1	7.335e-5	1	NC	4	NC	1
424			min	-.007	2	-.003	2	0	10	-1.939e-5	3	2619.248	3	NC	1
425		4	max	.007	3	.004	2	.002	1	5.851e-5	1	NC	4	NC	1
426			min	-.007	2	-.004	3	-.001	3	-2.744e-5	3	1870.676	3	NC	1
427		5	max	.007	3	.01	2	.002	1	4.368e-5	1	NC	4	NC	1
428			min	-.007	2	-.01	3	-.002	3	-3.549e-5	3	1514.54	3	9783.442	3
429		6	max	.007	3	.015	2	.002	1	2.884e-5	1	NC	4	NC	1
430			min	-.007	2	-.015	3	-.003	3	-4.354e-5	3	1316.555	3	8526.611	3
431		7	max	.007	3	.019	2	.001	1	1.4e-5	1	NC	4	NC	1
432			min	-.007	2	-.019	3	-.004	3	-5.16e-5	3	1196.168	2	7806.666	3
433		8	max	.007	3	.022	2	.001	1	-2.295e-7	10	NC	4	NC	1
434			min	-.007	2	-.021	3	-.004	3	-5.965e-5	3	1104.749	2	7414.802	3
435		9	max	.007	3	.024	2	0	1	1.261e-6	10	NC	4	NC	1
436			min	-.007	2	-.023	3	-.005	3	-6.77e-5	3	1050.623	2	7257.232	3
437		10	max	.007	3	.024	2	0	1	2.752e-6	10	NC	4	NC	1
438			min	-.007	2	-.023	3	-.005	3	-7.575e-5	3	1025.619	2	7294.756	3
439		11	max	.007	3	.024	2	0	10	4.243e-6	10	NC	4	NC	1
440			min	-.007	2	-.022	3	-.005	3	-8.381e-5	3	1026.797	2	7521.99	3
441		12	max	.007	3	.022	2	0	10	5.734e-6	10	NC	4	NC	1
442			min	-.007	2	-.02	3	-.004	3	-9.186e-5	3	1055.346	2	7963.407	3
443		13	max	.007	3	.02	2	0	10	7.225e-6	10	NC	4	NC	1
444			min	-.007	2	-.017	3	-.004	3	-9.991e-5	3	1117.233	2	8681.923	3
445		14	max	.007	3	.016	2	0	10	8.716e-6	10	NC	4	NC	1
446			min	-.007	2	-.014	3	-.003	3	-1.08e-4	3	1226.224	2	9806.761	3
447		15	max	.007	3	.01	2	0	10	1.021e-5	10	NC	4	NC	1
448			min	-.007	2	-.009	3	-.003	3	-1.16e-4	3	1412.529	2	NC	1
449		16	max	.007	3	.004	2	0	10	1.131e-5	10	NC	4	NC	1
450			min	-.007	2	-.003	3	-.002	3	-1.172e-4	3	1750.005	2	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.007	3	.003	3	0	10	4.462e-5	3	NC	4	NC	1
452		min	-.007	2	-.005	2	-.001	1	-4.946e-5	9	2474.638	2	NC	1
453	18	max	.007	3	.01	3	0	10	1.771e-3	3	NC	4	NC	1
454		min	-.007	2	-.014	2	0	9	-3.259e-3	2	4792.665	2	NC	1
455	19	max	.007	3	.018	3	0	3	3.526e-3	3	NC	1	NC	1
456		min	-.007	2	-.024	2	0	9	-6.575e-3	2	NC	1	NC	1
457	M13	1	max	0	.022	3	.007	3	3.594e-3	3	NC	1	NC	1
458		min	-.003	3	-.019	2	-.007	2	-3.088e-3	2	NC	1	NC	1
459	2	max	0	9	.052	3	.005	3	4.427e-3	3	NC	4	NC	1
460		min	-.003	3	-.04	2	-.007	2	-3.807e-3	2	2732.871	3	NC	1
461	3	max	0	9	.079	3	.005	3	5.259e-3	3	NC	4	NC	1
462		min	-.003	3	-.058	2	-.007	2	-4.525e-3	2	1476.257	3	NC	1
463	4	max	0	9	.097	3	.006	3	6.092e-3	3	NC	4	NC	1
464		min	-.003	3	-.072	2	-.008	2	-5.243e-3	2	1114.718	3	NC	1
465	5	max	0	9	.106	3	.008	3	6.925e-3	3	NC	4	NC	1
466		min	-.003	3	-.079	2	-.01	2	-5.961e-3	2	993.686	3	NC	1
467	6	max	0	9	.106	3	.01	3	7.758e-3	3	NC	4	NC	1
468		min	-.003	3	-.08	2	-.013	2	-6.68e-3	2	994.828	3	NC	1
469	7	max	0	9	.099	3	.013	3	8.591e-3	3	NC	4	NC	1
470		min	-.003	3	-.076	2	-.016	2	-7.398e-3	2	1096.122	3	NC	1
471	8	max	0	9	.086	3	.015	3	9.424e-3	3	NC	4	NC	1
472		min	-.003	3	-.068	2	-.019	2	-8.116e-3	2	1307.133	3	7222.954	2
473	9	max	0	9	.074	3	.018	3	1.026e-2	3	NC	4	NC	1
474		min	-.003	3	-.061	2	-.022	2	-8.834e-3	2	1614.45	3	5887.846	2
475	10	max	0	9	.068	3	.02	3	1.109e-2	3	NC	4	NC	4
476		min	-.003	3	-.057	2	-.023	2	-9.553e-3	2	1817.769	3	5458.303	2
477	11	max	0	9	.074	3	.021	3	1.026e-2	3	NC	4	NC	1
478		min	-.003	3	-.061	2	-.022	2	-8.834e-3	2	1614.449	3	5855.201	3
479	12	max	0	9	.086	3	.022	3	9.428e-3	3	NC	4	NC	1
480		min	-.003	3	-.068	2	-.019	2	-8.116e-3	2	1307.132	3	5762.353	3
481	13	max	0	9	.099	3	.021	3	8.598e-3	3	NC	4	NC	1
482		min	-.003	3	-.076	2	-.016	2	-7.398e-3	2	1096.121	3	6108.336	3
483	14	max	0	9	.107	3	.019	3	7.767e-3	3	NC	4	NC	1
484		min	-.003	3	-.08	2	-.013	2	-6.68e-3	2	994.827	3	6957.784	3
485	15	max	0	9	.107	3	.017	3	6.937e-3	3	NC	4	NC	1
486		min	-.003	3	-.079	2	-.01	2	-5.962e-3	2	993.685	3	8595.279	3
487	16	max	0	9	.098	3	.014	3	6.106e-3	3	NC	4	NC	1
488		min	-.003	3	-.072	2	-.008	2	-5.243e-3	2	1114.717	3	NC	1
489	17	max	0	9	.079	3	.011	3	5.276e-3	3	NC	4	NC	1
490		min	-.003	3	-.058	2	-.007	2	-4.525e-3	2	1476.256	3	NC	1
491	18	max	0	9	.053	3	.009	3	4.445e-3	3	NC	4	NC	1
492		min	-.003	3	-.04	2	-.007	2	-3.807e-3	2	2732.87	3	NC	1
493	19	max	0	9	.022	3	.007	3	3.614e-3	3	NC	1	NC	1
494		min	-.003	3	-.019	2	-.007	2	-3.089e-3	2	NC	1	NC	1
495	M16	1	max	0	.018	3	.007	3	3.824e-3	2	NC	1	NC	1
496		min	0	3	-.024	2	-.007	2	-2.818e-3	3	NC	1	NC	1
497	2	max	0	9	.034	3	.009	3	4.715e-3	2	NC	4	NC	1
498		min	0	3	-.054	2	-.007	2	-3.434e-3	3	2821.419	2	NC	1
499	3	max	0	9	.048	3	.011	3	5.606e-3	2	NC	4	NC	1
500		min	0	3	-.079	2	-.007	2	-4.05e-3	3	1519.651	2	NC	1
501	4	max	0	9	.058	3	.014	3	6.498e-3	2	NC	4	NC	1
502		min	0	3	-.098	2	-.008	2	-4.667e-3	3	1141.714	2	NC	1
503	5	max	0	9	.064	3	.016	3	7.389e-3	2	NC	4	NC	1
504		min	0	3	-.107	2	-.01	2	-5.283e-3	3	1009.842	2	9242.149	3
505	6	max	0	9	.066	3	.018	3	8.28e-3	2	NC	4	NC	1
506		min	0	3	-.108	2	-.013	2	-5.899e-3	3	999.193	2	7671.601	3
507	7	max	0	9	.064	3	.019	3	9.172e-3	2	NC	4	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.102	2	-.016	2	-6.516e-3	3	1081.706	2	6815.64	3
509	8	max	0	9	.059	3	.02	3	1.006e-2	2	NC	4	NC	1
510		min	0	3	-.091	2	-.019	2	-7.132e-3	3	1257.149	2	6410.501	3
511	9	max	0	9	.055	3	.02	3	1.095e-2	2	NC	4	NC	4
512		min	0	3	-.08	2	-.022	2	-7.748e-3	3	1503.113	2	5877.251	2
513	10	max	0	9	.052	3	.019	3	1.185e-2	2	NC	4	NC	4
514		min	0	3	-.075	2	-.023	2	-8.365e-3	3	1658.949	2	5449.007	2
515	11	max	0	9	.055	3	.018	3	1.095e-2	2	NC	4	NC	4
516		min	0	3	-.08	2	-.022	2	-7.746e-3	3	1503.113	2	5877.26	2
517	12	max	0	9	.059	3	.017	3	1.006e-2	2	NC	4	NC	1
518		min	0	3	-.091	2	-.019	2	-7.128e-3	3	1257.149	2	7207.94	2
519	13	max	0	9	.064	3	.015	3	9.172e-3	2	NC	4	NC	1
520		min	0	3	-.102	2	-.016	2	-6.509e-3	3	1081.706	2	9635.039	3
521	14	max	0	9	.066	3	.014	3	8.281e-3	2	NC	4	NC	1
522		min	0	3	-.108	2	-.013	2	-5.891e-3	3	999.193	2	NC	1
523	15	max	0	9	.064	3	.012	3	7.39e-3	2	NC	4	NC	1
524		min	0	3	-.107	2	-.01	2	-5.273e-3	3	1009.842	2	NC	1
525	16	max	0	9	.058	3	.01	3	6.499e-3	2	NC	4	NC	1
526		min	0	3	-.098	2	-.008	2	-4.654e-3	3	1141.714	2	NC	1
527	17	max	0	9	.048	3	.009	3	5.608e-3	2	NC	4	NC	1
528		min	0	3	-.079	2	-.007	2	-4.036e-3	3	1519.651	2	NC	1
529	18	max	0	9	.034	3	.008	3	4.716e-3	2	NC	4	NC	1
530		min	0	3	-.054	2	-.007	2	-3.417e-3	3	2821.419	2	NC	1
531	19	max	0	9	.018	3	.007	3	3.825e-3	2	NC	1	NC	1
532		min	0	3	-.024	2	-.007	2	-2.799e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	3.745e-4	3	NC	1	NC	1
534		min	0	1	0	0	0	1	-3.748e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.385e-4	3	NC	1	NC	1
536		min	0	2	-.002	4	0	3	-3.946e-4	2	NC	1	NC	1
537	3	max	0	3	0	15	.002	1	1.103e-3	3	NC	1	NC	1
538		min	0	2	-.003	4	-.003	3	-7.518e-4	2	NC	1	NC	1
539	4	max	0	3	-.001	15	.005	1	1.467e-3	3	NC	1	NC	4
540		min	0	2	-.005	4	-.006	3	-1.109e-3	2	NC	1	5509.936	3
541	5	max	0	3	-.002	15	.008	2	1.831e-3	3	NC	1	NC	4
542		min	0	2	-.006	4	-.01	3	-1.466e-3	2	8712.469	4	3613.336	3
543	6	max	0	3	-.002	15	.011	2	2.195e-3	3	NC	3	NC	4
544		min	0	2	-.008	4	-.014	3	-1.823e-3	2	7332.463	4	2629.444	3
545	7	max	0	3	-.002	15	.015	2	2.559e-3	3	NC	3	NC	4
546		min	-.001	2	-.009	4	-.019	3	-2.18e-3	2	6502.572	4	2054.632	3
547	8	max	0	3	-.002	15	.018	2	2.923e-3	3	NC	3	NC	4
548		min	-.001	2	-.009	4	-.024	3	-2.538e-3	2	6004.513	4	1693.512	3
549	9	max	0	3	-.002	15	.022	2	3.287e-3	3	NC	5	NC	4
550		min	-.002	2	-.01	4	-.028	3	-2.895e-3	2	5736.428	4	1457.288	3
551	10	max	0	3	-.002	15	.024	2	3.651e-3	3	NC	5	NC	4
552		min	-.002	2	-.01	4	-.031	3	-3.252e-3	2	5651.618	4	1301.399	3
553	11	max	0	3	-.002	15	.026	2	4.015e-3	3	NC	5	NC	4
554		min	-.002	2	-.01	4	-.033	3	-3.609e-3	2	5736.428	4	1202.46	3
555	12	max	0	3	-.002	15	.026	2	4.379e-3	3	NC	3	NC	4
556		min	-.002	2	-.009	4	-.034	3	-3.966e-3	2	6004.513	4	1148.861	3
557	13	max	.001	3	-.002	15	.025	2	4.743e-3	3	NC	3	NC	4
558		min	-.002	2	-.009	4	-.033	3	-4.323e-3	2	6502.572	4	1137.208	3
559	14	max	.001	3	-.002	15	.023	1	5.107e-3	3	NC	3	NC	4
560		min	-.003	2	-.008	4	-.03	3	-4.68e-3	2	7332.463	4	1172.426	3
561	15	max	.001	3	-.001	2	.019	1	5.471e-3	3	NC	1	NC	4
562		min	-.003	2	-.007	4	-.025	3	-5.038e-3	2	8712.469	4	1272.699	3
563	16	max	.001	3	0	2	.014	1	5.836e-3	3	NC	1	NC	4
564		min	-.003	2	-.005	4	-.017	3	-5.395e-3	2	NC	1	1487.449	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.001	3	.002	2	.006	1	6.2e-3	3	NC	1	NC	4
566			min	-.003	2	-.004	4	-.007	3	-5.752e-3	2	NC	1	1971.769	3
567		18	max	.002	3	.003	2	.007	3	6.564e-3	3	NC	1	NC	4
568			min	-.003	2	-.002	4	-.009	2	-6.109e-3	2	NC	1	3510.259	3
569		19	max	.002	3	.005	2	.024	3	6.928e-3	3	NC	1	NC	1
570			min	-.004	2	0	9	-.024	2	-6.466e-3	2	NC	1	NC	1
571	M16A	1	max	.001	2	.002	2	.008	3	1.995e-3	3	NC	1	NC	1
572			min	-.002	3	-.001	3	-.008	2	-2.076e-3	2	NC	1	NC	1
573		2	max	.001	2	0	2	.001	3	1.919e-3	3	NC	1	NC	1
574			min	-.002	3	-.002	4	-.003	2	-1.98e-3	2	NC	1	9670.084	3
575		3	max	0	2	0	15	.003	1	1.843e-3	3	NC	1	NC	4
576			min	-.001	3	-.004	4	-.003	3	-1.883e-3	2	NC	1	5472.584	3
577		4	max	0	2	-.001	15	.005	1	1.768e-3	3	NC	1	NC	4
578			min	-.001	3	-.005	4	-.007	3	-1.787e-3	2	NC	1	4163.396	3
579		5	max	0	2	-.002	15	.007	1	1.692e-3	3	NC	1	NC	4
580			min	-.001	3	-.007	4	-.01	3	-1.691e-3	2	8712.469	4	3596.776	3
581		6	max	0	2	-.002	15	.008	1	1.616e-3	3	NC	3	NC	4
582			min	-.001	3	-.008	4	-.011	3	-1.595e-3	2	7332.463	4	3350.305	3
583		7	max	0	2	-.002	15	.008	1	1.54e-3	3	NC	3	NC	4
584			min	-.001	3	-.009	4	-.012	3	-1.499e-3	2	6502.572	4	3291.794	3
585		8	max	0	2	-.002	15	.008	1	1.464e-3	3	NC	3	NC	4
586			min	0	3	-.009	4	-.012	3	-1.403e-3	2	6004.513	4	3376.331	3
587		9	max	0	2	-.002	15	.008	1	1.388e-3	3	NC	5	NC	4
588			min	0	3	-.01	4	-.011	3	-1.307e-3	2	5736.428	4	3598.44	3
589		10	max	0	2	-.002	15	.007	1	1.312e-3	3	NC	5	NC	4
590			min	0	3	-.01	4	-.01	3	-1.211e-3	2	5651.618	4	3981.234	3
591		11	max	0	2	-.002	15	.006	1	1.236e-3	3	NC	5	NC	4
592			min	0	3	-.01	4	-.009	3	-1.115e-3	2	5736.428	4	4581.761	3
593		12	max	0	2	-.002	15	.005	1	1.16e-3	3	NC	3	NC	4
594			min	0	3	-.009	4	-.007	3	-1.019e-3	2	6004.513	4	5513.602	3
595		13	max	0	2	-.002	15	.004	1	1.084e-3	3	NC	3	NC	1
596			min	0	3	-.009	4	-.005	3	-9.228e-4	2	6502.572	4	7005.072	3
597		14	max	0	2	-.002	15	.003	1	1.008e-3	3	NC	3	NC	1
598			min	0	3	-.008	4	-.004	3	-8.267e-4	2	7332.463	4	9556.07	3
599		15	max	0	2	-.002	15	.001	1	9.325e-4	3	NC	1	NC	1
600			min	0	3	-.006	4	-.002	3	-7.307e-4	2	8712.469	4	NC	1
601		16	max	0	2	-.001	15	0	4	8.565e-4	3	NC	1	NC	1
602			min	0	3	-.005	4	0	3	-6.346e-4	2	NC	1	NC	1
603		17	max	0	2	0	15	0	4	7.806e-4	3	NC	1	NC	1
604			min	0	3	-.003	4	0	2	-5.385e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	4	7.047e-4	3	NC	1	NC	1
606			min	0	3	-.002	4	0	2	-4.425e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	6.288e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.464e-4	2	NC	1	NC	1



**Anchor Designer™**  
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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

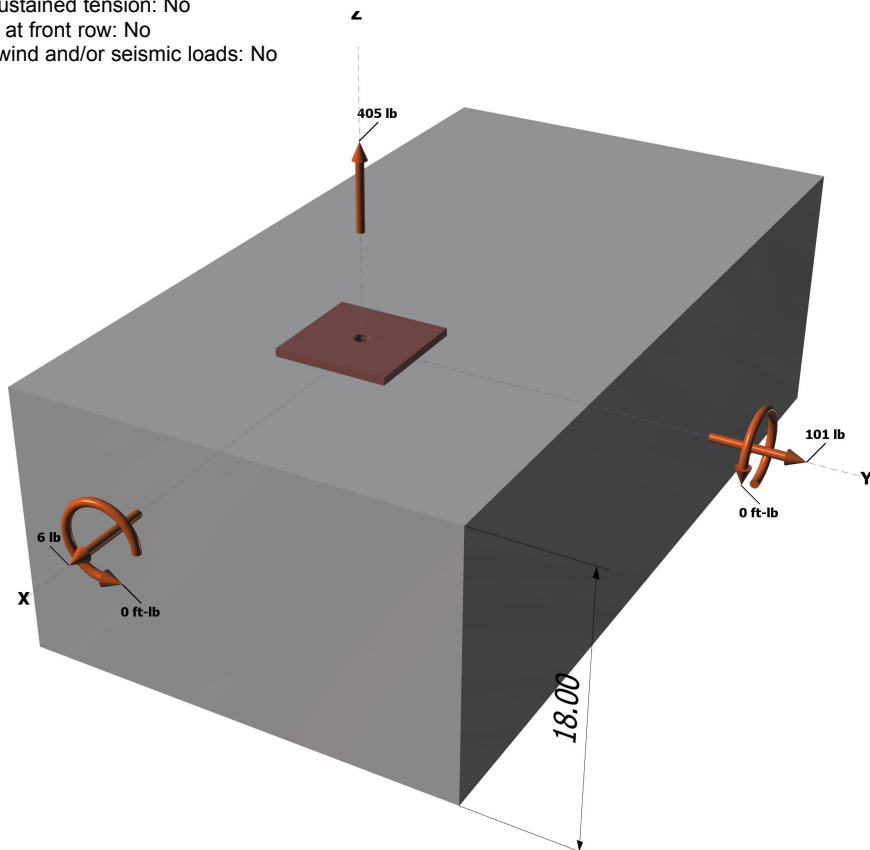
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

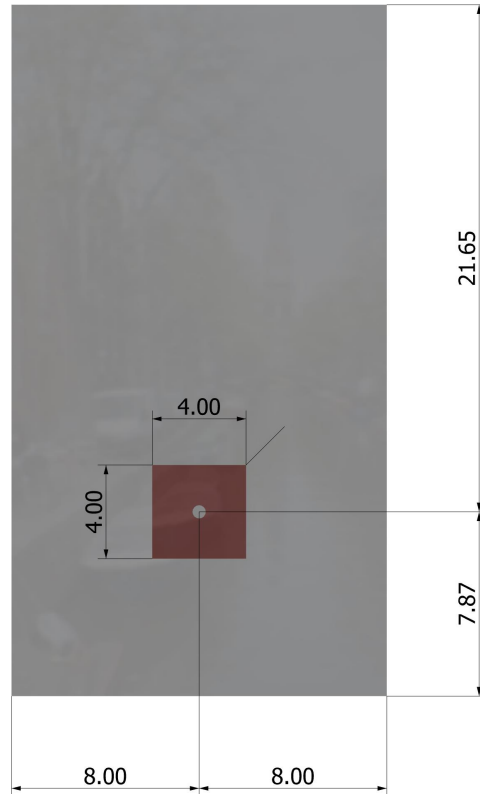
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Engineer:	HCV	Page:	1/5
Project:	Standard PVMINI - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

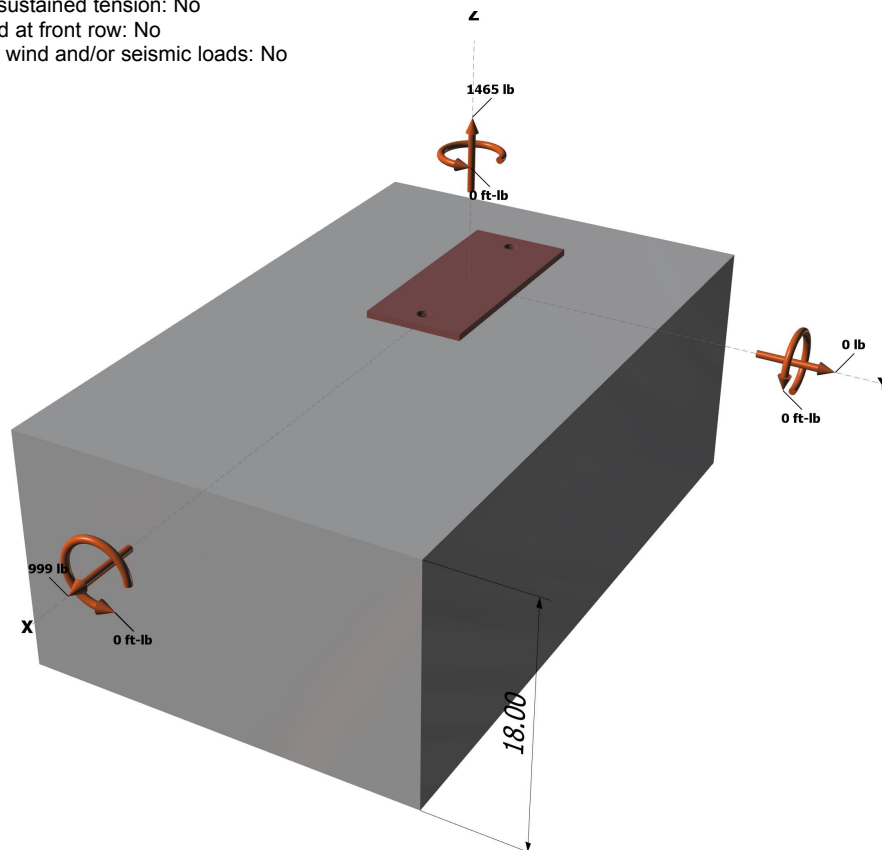
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



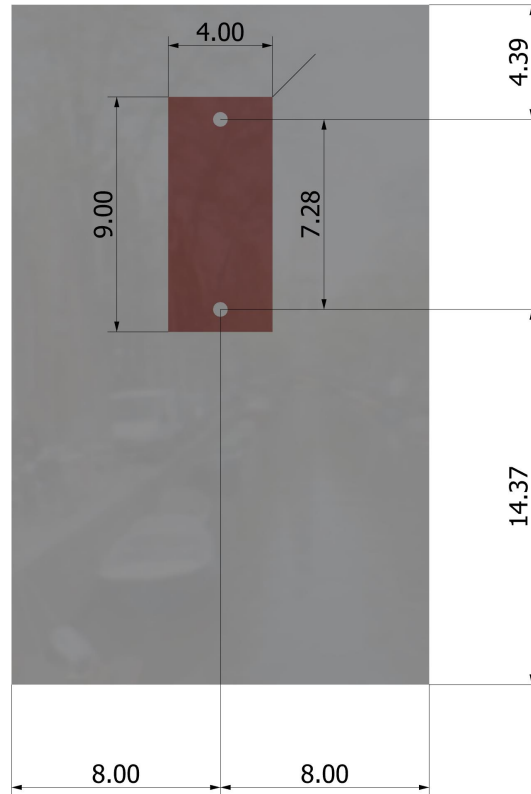
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1465  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.