

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	85 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 11.34 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

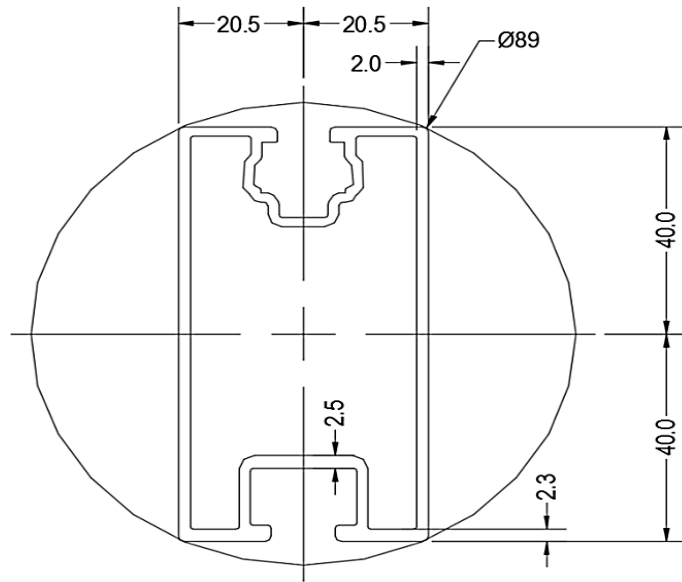
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

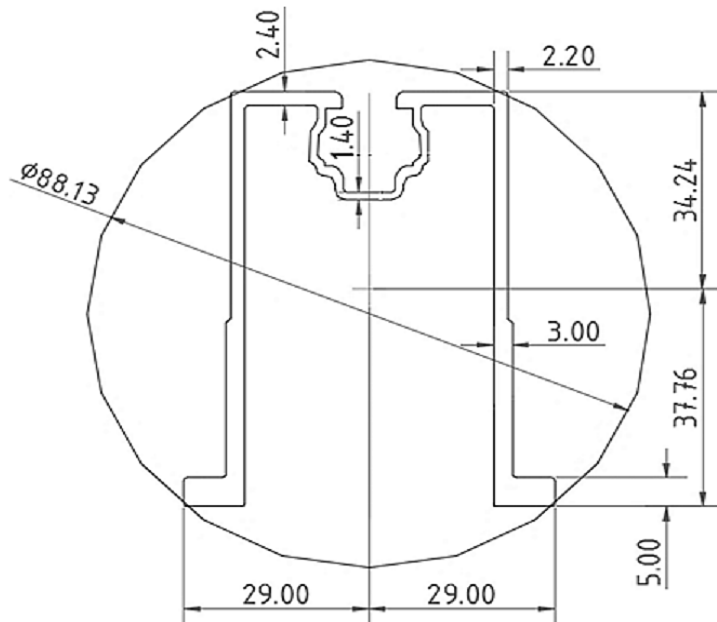
Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	28.61 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	1.139 k-ft
$M_z$ =	0.264 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>96%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

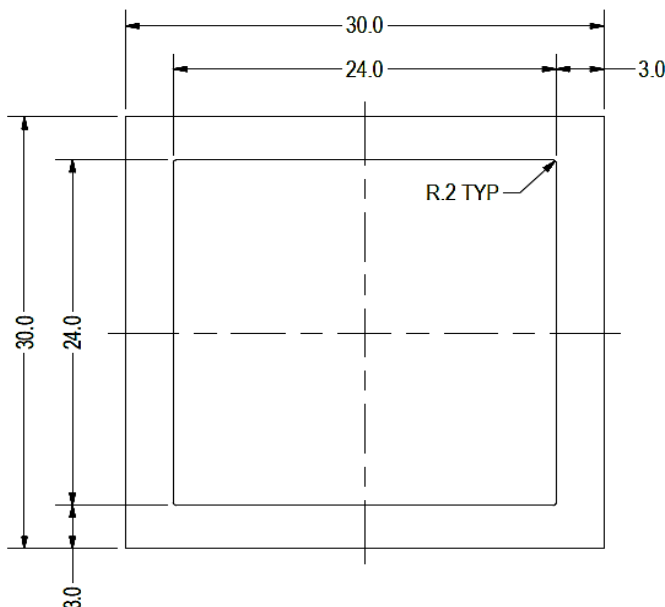
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	30.05 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.190 k-ft
$M_z$ =	-0.151 k-ft
$P_n$ =	0.063 k
$M_{y \text{ allowable}}$ =	1.474 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>43%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

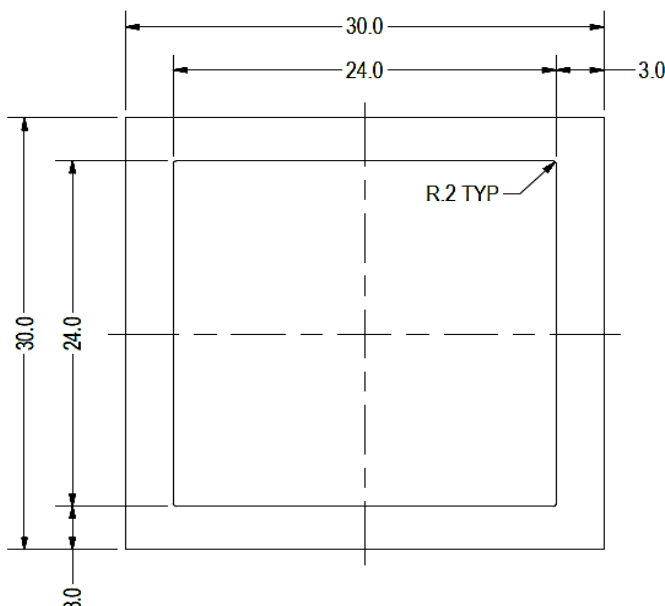
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	1.445 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>12%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.473 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>12%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.204 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<u>21%</u>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.007 k-ft
$P_n$ =	0.046 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>16%</u>



A cross brace kit is required every 12 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

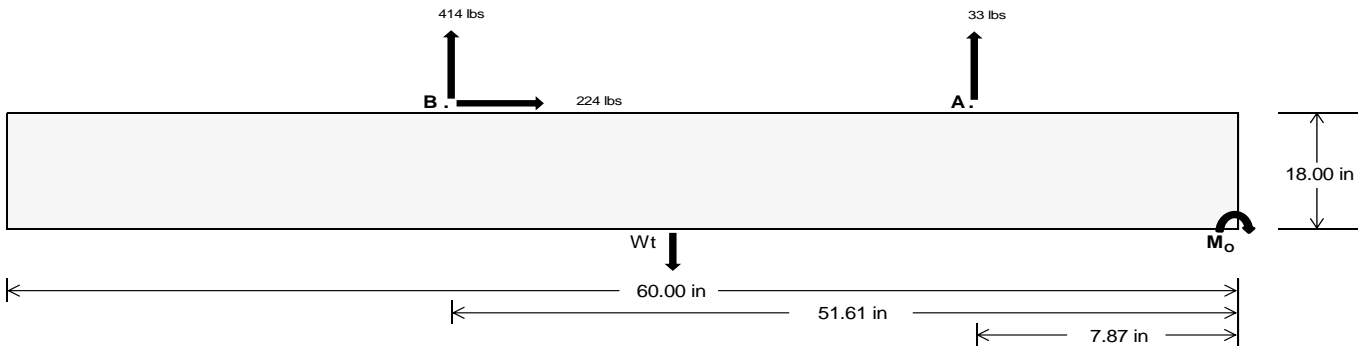
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>144.91</u>	<u>1725.77</u>	k
Compressive Load =	<u>1878.19</u>	<u>1544.65</u>	k
Lateral Load =	<u>6.04</u>	<u>930.46</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 25649.2$  in-lbs  
Resisting Force Required = 854.97 lbs  
S.F. = 1.67  
Weight Required = 1424.95 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 223.55 lbs  
Friction = 0.4  
Weight Required = 558.87 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 223.55 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$   
22 in 23 in 24 in 25 in  
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	738 lbs	738 lbs	738 lbs	738 lbs	510 lbs	510 lbs	510 lbs	510 lbs	876 lbs	876 lbs	876 lbs	876 lbs	-67 lbs	-67 lbs	-67 lbs	-67 lbs
$F_B$	534 lbs	534 lbs	534 lbs	534 lbs	549 lbs	549 lbs	549 lbs	549 lbs	768 lbs	768 lbs	768 lbs	768 lbs	-828 lbs	-828 lbs	-828 lbs	-828 lbs
$F_V$	80 lbs	80 lbs	80 lbs	80 lbs	408 lbs	408 lbs	408 lbs	408 lbs	360 lbs	360 lbs	360 lbs	360 lbs	-447 lbs	-447 lbs	-447 lbs	-447 lbs
$P_{total}$	3266 lbs	3357 lbs	3448 lbs	3538 lbs	3053 lbs	3144 lbs	3234 lbs	3325 lbs	3638 lbs	3729 lbs	3819 lbs	3910 lbs	302 lbs	356 lbs	410 lbs	465 lbs
$M$	519 lbs-ft	519 lbs-ft	519 lbs-ft	519 lbs-ft	563 lbs-ft	563 lbs-ft	563 lbs-ft	563 lbs-ft	772 lbs-ft	772 lbs-ft	772 lbs-ft	772 lbs-ft	697 lbs-ft	697 lbs-ft	697 lbs-ft	697 lbs-ft
$e$	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	0.21 ft	0.21 ft	0.20 ft	0.20 ft	2.31 ft	1.96 ft	1.70 ft	1.50 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	288.3 psf	285.3 psf	282.4 psf	279.8 psf	259.4 psf	257.6 psf	255.9 psf	254.4 psf	295.8 psf	292.4 psf	289.3 psf	286.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	424.3 psf	415.3 psf	407.1 psf	399.5 psf	406.7 psf	398.5 psf	391.0 psf	384.0 psf	498.0 psf	485.8 psf	474.6 psf	464.4 psf	580.1 psf	228.5 psf	170.7 psf	148.7 psf

Maximum Bearing Pressure = 580 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

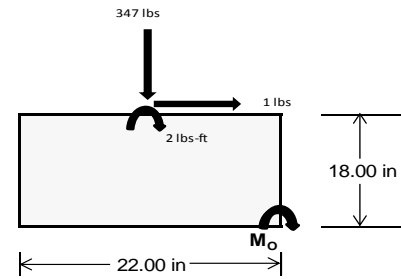
#### Overturning Check

$M_o = 314.2 \text{ ft-lbs}$   
 Resisting Force Required = 342.75 lbs  
 S.F. = 1.67  
 Weight Required = 571.25 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	96 lbs	259 lbs	91 lbs	352 lbs	1044 lbs	347 lbs	28 lbs	76 lbs	27 lbs
$F_v$	6 lbs	6 lbs	0 lbs	27 lbs	26 lbs	1 lbs	2 lbs	2 lbs	0 lbs
$P_{total}$	2564 lbs	2727 lbs	2559 lbs	2702 lbs	3394 lbs	2697 lbs	750 lbs	797 lbs	748 lbs
$M$	10 lbs-ft	9 lbs-ft	0 lbs-ft	46 lbs-ft	39 lbs-ft	4 lbs-ft	3 lbs-ft	3 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.02 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.80 ft	1.81 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	276.0 sqft	294.3 sqft	279.0 sqft	278.4 sqft	356.4 sqft	292.8 sqft	80.8 sqft	86.1 sqft	81.6 sqft
$f_{max}$	283.4 psf	300.7 psf	279.3 psf	311.1 psf	384.1 psf	295.6 psf	82.8 psf	87.9 psf	81.7 psf



Maximum Bearing Pressure = 384 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



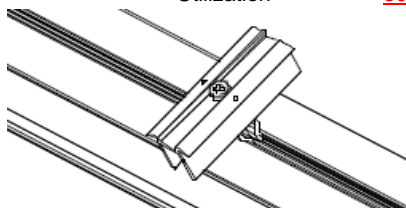
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

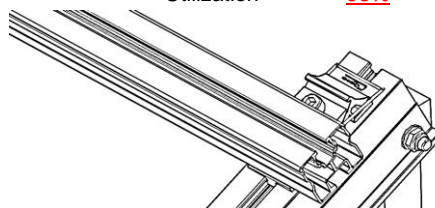
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.368 k
Allowable Uplift =	1.214 k
Utilization =	<u>30%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.099 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

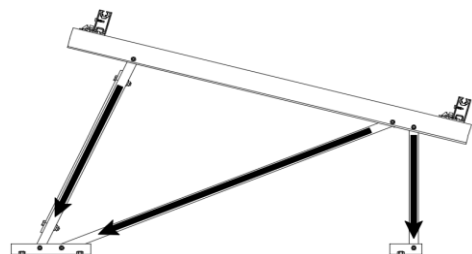
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.445 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

#### Diagonal Strut

Maximum Axial Load =	0.473 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



#### Rear Strut

Maximum Axial Load =	1.204 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

#### Bracing

Maximum Axial Load =	0.046 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	30.83 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.617 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 37.95$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 38.1$$

$$m = 0.63$$

$$C_0 = 40.784$$

$$Cc = 39.216$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 79.7$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.6 \text{ ksi}$$

$$I_x = 498305 \text{ mm}^4$$

$$1.197 \text{ in}^4$$

$$y = 40.784 \text{ mm}$$

$$S_x = 0.746 \text{ in}^3$$

$$M_{\max} St = 1.778 \text{ k-ft}$$

### 3.4.18

$$h/t = 6.6$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20.5$$

$$Cc = 20.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 22.7 \text{ ksi}$$

$$I_y = 148662 \text{ mm}^4$$

$$0.357 \text{ in}^4$$

$$x = 20.5 \text{ mm}$$

$$S_y = 0.443 \text{ in}^3$$

$$M_{\max} Wk = 0.838 \text{ k-ft}$$

### Compression

#### 3.4.9

$$b/t = 6.6$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$
  

$$b/t = 37.95$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = (\phi k_2 \sqrt{(BpE)}) / (1.6b/t)$$

$$\phi F_L = 21.4 \text{ ksi}$$

#### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 21.42 \text{ ksi}$$

$$A = 620.02 \text{ mm}^2$$

$$0.96 \text{ in}^2$$

$$P_{\max} = 20.59 \text{ kips}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.55 \\ &19.7698 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 30.0 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.55 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 30.0 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.474 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 1.98863$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.85841$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 7.59722 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 7.60 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 3.81 \text{ kips}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

### 3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{max} = 5.82 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	168.821	2	318.028	1	-.003	15	0	2	0	1	0	1
2		min	-223.864	3	-404.278	3	-.254	1	0	3	0	1	0	1
3	N7	max	0	15	531.915	1	-.09	15	0	15	0	1	0	1
4		min	-.2	1	-23.813	3	-2.137	1	-.004	1	0	1	0	1
5	N15	max	-.001	15	1444.765	1	.702	1	.001	1	0	1	0	1
6		min	-2.11	1	-111.47	3	-.294	3	0	3	0	1	0	1
7	N16	max	682.241	2	1188.194	1	-.34	10	0	1	0	1	0	1
8		min	-715.742	3	-1327.513	3	-42.75	1	0	3	0	1	0	1
9	N23	max	0	15	531.593	1	4.644	1	.008	1	0	1	0	1
10		min	-.2	1	-23.308	3	.185	15	0	15	0	1	0	1
11	N24	max	169.392	2	324.058	1	39.795	1	.002	1	0	1	0	1
12		min	-223.922	3	-401.234	3	.051	10	0	12	0	1	0	1
13	Totals:	max	1018.235	2	4338.552	1	0	10						
14		min	-1163.684	3	-2291.616	3	0	1						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	366.645	1	.64	4	.807	1	0	15	0	3	0	1
2			min	-364.515	3	.151	15	-.024	3	-.001	1	0	1	0	1
3		2	max	366.761	1	.595	4	.807	1	0	15	0	12	0	15
4			min	-364.428	3	.14	15	-.024	3	-.001	1	0	1	0	4
5		3	max	366.878	1	.549	4	.807	1	0	15	0	1	0	15
6			min	-364.341	3	.13	15	-.024	3	-.001	1	0	3	0	4
7		4	max	366.994	1	.503	4	.807	1	0	15	0	1	0	15
8			min	-364.253	3	.119	15	-.024	3	-.001	1	0	3	0	4
9		5	max	367.111	1	.458	4	.807	1	0	15	0	1	0	15
10			min	-364.166	3	.108	15	-.024	3	-.001	1	0	3	0	4
11		6	max	367.227	1	.412	4	.807	1	0	15	0	1	0	15
12			min	-364.079	3	.098	15	-.024	3	-.001	1	0	3	0	4
13		7	max	367.343	1	.366	4	.807	1	0	15	0	1	0	15
14			min	-363.992	3	.087	15	-.024	3	-.001	1	0	3	0	4
15		8	max	367.46	1	.321	4	.807	1	0	15	0	1	0	15
16			min	-363.904	3	.076	15	-.024	3	-.001	1	0	3	0	4
17		9	max	367.576	1	.275	4	.807	1	0	15	0	1	0	15
18			min	-363.817	3	.065	15	-.024	3	-.001	1	0	3	0	4
19		10	max	367.693	1	.229	4	.807	1	0	15	0	1	0	15
20			min	-363.73	3	.055	15	-.024	3	-.001	1	0	3	0	4
21		11	max	367.809	1	.184	4	.807	1	0	15	.001	1	0	15
22			min	-363.642	3	.044	15	-.024	3	-.001	1	0	3	0	4
23		12	max	367.925	1	.138	4	.807	1	0	15	.001	1	0	15
24			min	-363.555	3	.033	15	-.024	3	-.001	1	0	3	0	4
25		13	max	368.042	1	.098	2	.807	1	0	15	.001	1	0	15
26			min	-363.468	3	.017	12	-.024	3	-.001	1	0	3	0	4
27		14	max	368.158	1	.062	2	.807	1	0	15	.002	1	0	15
28			min	-363.38	3	-.003	3	-.024	3	-.001	1	0	3	0	4
29		15	max	368.275	1	.027	2	.807	1	0	15	.002	1	0	15
30			min	-363.293	3	-.03	3	-.024	3	-.001	1	0	3	0	4
31		16	max	368.391	1	-.008	10	.807	1	0	15	.002	1	0	15
32			min	-363.206	3	-.056	3	-.024	3	-.001	1	0	3	0	4
33		17	max	368.507	1	-.021	15	.807	1	0	15	.002	1	0	15
34			min	-363.118	3	-.09	4	-.024	3	-.001	1	0	3	0	4
35		18	max	368.624	1	-.031	15	.807	1	0	15	.002	1	0	15
36			min	-363.031	3	-.136	4	-.024	3	-.001	1	0	3	0	4
37		19	max	368.74	1	-.042	15	.807	1	0	15	.002	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-362.944	3	-.182	4	-.024	3	-.001	1	0	3	0	4
39	M3	1	max	100.045	2	1.776	4	-.032	15	0	.003	1	0	4
40		min	-130.753	3	.418	15	-.856	1	0	1	0	15	0	15
41		2	max	99.977	2	1.599	4	-.032	15	0	.002	1	0	2
42		min	-130.804	3	.376	15	-.856	1	0	1	0	15	0	12
43		3	max	99.908	2	1.422	4	-.032	15	0	.002	1	0	2
44		min	-130.856	3	.335	15	-.856	1	0	1	0	15	0	3
45		4	max	99.839	2	1.244	4	-.032	15	0	.002	1	0	15
46		min	-130.907	3	.293	15	-.856	1	0	1	0	15	0	4
47		5	max	99.771	2	1.067	4	-.032	15	0	.002	1	0	15
48		min	-130.959	3	.251	15	-.856	1	0	1	0	15	0	4
49		6	max	99.702	2	.89	4	-.032	15	0	.002	1	0	15
50		min	-131.01	3	.21	15	-.856	1	0	1	0	15	0	4
51		7	max	99.634	2	.713	4	-.032	15	0	.002	1	0	15
52		min	-131.062	3	.168	15	-.856	1	0	1	0	15	0	4
53		8	max	99.565	2	.536	4	-.032	15	0	.001	1	0	15
54		min	-131.113	3	.126	15	-.856	1	0	1	0	15	-.001	4
55		9	max	99.496	2	.358	4	-.032	15	0	.001	1	0	15
56		min	-131.164	3	.085	15	-.856	1	0	1	0	15	-.001	4
57		10	max	99.428	2	.181	4	-.032	15	0	.001	1	0	15
58		min	-131.216	3	.043	15	-.856	1	0	1	0	15	-.001	4
59		11	max	99.359	2	.024	2	-.032	15	0	0	1	0	15
60		min	-131.267	3	-.022	3	-.856	1	0	1	0	15	-.001	4
61		12	max	99.291	2	-.04	15	-.032	15	0	0	1	0	15
62		min	-131.319	3	-.173	4	-.856	1	0	1	0	15	-.001	4
63		13	max	99.222	2	-.082	15	-.032	15	0	0	1	0	15
64		min	-131.37	3	-.35	4	-.856	1	0	1	0	15	-.001	4
65		14	max	99.153	2	-.123	15	-.032	15	0	0	1	0	15
66		min	-131.422	3	-.528	4	-.856	1	0	1	0	12	-.001	4
67		15	max	99.085	2	-.165	15	-.032	15	0	0	1	0	15
68		min	-131.473	3	-.705	4	-.856	1	0	1	0	12	0	4
69		16	max	99.016	2	-.207	15	-.032	15	0	0	15	0	15
70		min	-131.525	3	-.882	4	-.856	1	0	1	0	1	0	4
71		17	max	98.948	2	-.248	15	-.032	15	0	0	15	0	15
72		min	-131.576	3	-1.059	4	-.856	1	0	1	0	1	0	4
73		18	max	98.879	2	-.29	15	-.032	15	0	0	15	0	15
74		min	-131.628	3	-1.237	4	-.856	1	0	1	0	1	0	4
75		19	max	98.81	2	-.332	15	-.032	15	0	0	15	0	1
76		min	-131.679	3	-1.414	4	-.856	1	0	1	0	1	0	1
77	M4	1	max	530.75	1	0	1	-.09	15	0	0	3	0	1
78		min	-24.687	3	0	1	-2.344	1	0	1	0	1	0	1
79		2	max	530.815	1	0	1	-.09	15	0	0	12	0	1
80		min	-24.638	3	0	1	-2.344	1	0	1	0	1	0	1
81		3	max	530.88	1	0	1	-.09	15	0	0	15	0	1
82		min	-24.59	3	0	1	-2.344	1	0	1	0	1	0	1
83		4	max	530.944	1	0	1	-.09	15	0	0	15	0	1
84		min	-24.541	3	0	1	-2.344	1	0	1	0	1	0	1
85		5	max	531.009	1	0	1	-.09	15	0	0	15	0	1
86		min	-24.492	3	0	1	-2.344	1	0	1	0	1	0	1
87		6	max	531.074	1	0	1	-.09	15	0	0	15	0	1
88		min	-24.444	3	0	1	-2.344	1	0	1	-.001	1	0	1
89		7	max	531.139	1	0	1	-.09	15	0	0	15	0	1
90		min	-24.395	3	0	1	-2.344	1	0	1	-.001	1	0	1
91		8	max	531.203	1	0	1	-.09	15	0	0	15	0	1
92		min	-24.347	3	0	1	-2.344	1	0	1	-.002	1	0	1
93		9	max	531.268	1	0	1	-.09	15	0	0	15	0	1
94		min	-24.298	3	0	1	-2.344	1	0	1	-.002	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	531.333	1	0	1	-.09	15	0	1	0	15	0	1
96		min	-24.25	3	0	1	-2.344	1	0	1	-.002	1	0	1
97	11	max	531.397	1	0	1	-.09	15	0	1	0	15	0	1
98		min	-24.201	3	0	1	-2.344	1	0	1	-.002	1	0	1
99	12	max	531.462	1	0	1	-.09	15	0	1	0	15	0	1
100		min	-24.153	3	0	1	-2.344	1	0	1	-.002	1	0	1
101	13	max	531.527	1	0	1	-.09	15	0	1	0	15	0	1
102		min	-24.104	3	0	1	-2.344	1	0	1	-.003	1	0	1
103	14	max	531.591	1	0	1	-.09	15	0	1	0	15	0	1
104		min	-24.056	3	0	1	-2.344	1	0	1	-.003	1	0	1
105	15	max	531.656	1	0	1	-.09	15	0	1	0	15	0	1
106		min	-24.007	3	0	1	-2.344	1	0	1	-.003	1	0	1
107	16	max	531.721	1	0	1	-.09	15	0	1	0	15	0	1
108		min	-23.959	3	0	1	-2.344	1	0	1	-.003	1	0	1
109	17	max	531.786	1	0	1	-.09	15	0	1	0	15	0	1
110		min	-23.91	3	0	1	-2.344	1	0	1	-.003	1	0	1
111	18	max	531.85	1	0	1	-.09	15	0	1	0	15	0	1
112		min	-23.862	3	0	1	-2.344	1	0	1	-.004	1	0	1
113	19	max	531.915	1	0	1	-.09	15	0	1	0	15	0	1
114		min	-23.813	3	0	1	-2.344	1	0	1	-.004	1	0	1
115	M6	1	max	1202.234	1	.641	.23	1	0	1	0	3	0	1
116		min	-1192.156	3	.151	15	-.112	3	0	15	0	2	0	1
117	2	max	1202.35	1	.595	4	.23	1	0	1	0	3	0	15
118		min	-1192.069	3	.14	15	-.112	3	0	15	0	2	0	4
119	3	max	1202.467	1	.55	4	.23	1	0	1	0	1	0	15
120		min	-1191.981	3	.13	15	-.112	3	0	15	0	15	0	4
121	4	max	1202.583	1	.504	4	.23	1	0	1	0	1	0	15
122		min	-1191.894	3	.119	15	-.112	3	0	15	0	15	0	4
123	5	max	1202.7	1	.458	4	.23	1	0	1	0	1	0	15
124		min	-1191.807	3	.108	15	-.112	3	0	15	0	12	0	4
125	6	max	1202.816	1	.414	2	.23	1	0	1	0	1	0	15
126		min	-1191.719	3	.098	15	-.112	3	0	15	0	3	0	4
127	7	max	1202.932	1	.378	2	.23	1	0	1	0	1	0	15
128		min	-1191.632	3	.082	12	-.112	3	0	15	0	3	0	4
129	8	max	1203.049	1	.343	2	.23	1	0	1	0	1	0	15
130		min	-1191.545	3	.064	12	-.112	3	0	15	0	3	0	4
131	9	max	1203.165	1	.307	2	.23	1	0	1	0	1	0	15
132		min	-1191.457	3	.046	12	-.112	3	0	15	0	3	0	4
133	10	max	1203.282	1	.271	2	.23	1	0	1	0	1	0	15
134		min	-1191.37	3	.029	12	-.112	3	0	15	0	3	0	4
135	11	max	1203.398	1	.236	2	.23	1	0	1	0	1	0	15
136		min	-1191.283	3	.01	3	-.112	3	0	15	0	3	0	2
137	12	max	1203.514	1	.2	2	.23	1	0	1	0	1	0	12
138		min	-1191.196	3	-.017	3	-.112	3	0	15	0	3	0	2
139	13	max	1203.631	1	.165	2	.23	1	0	1	0	1	0	12
140		min	-1191.108	3	-.043	3	-.112	3	0	15	0	3	0	2
141	14	max	1203.747	1	.129	2	.23	1	0	1	0	1	0	12
142		min	-1191.021	3	-.07	3	-.112	3	0	15	0	3	0	2
143	15	max	1203.864	1	.094	2	.23	1	0	1	0	1	0	12
144		min	-1190.934	3	-.097	3	-.112	3	0	15	0	3	0	2
145	16	max	1203.98	1	.058	2	.23	1	0	1	0	1	0	12
146		min	-1190.846	3	-.123	3	-.112	3	0	15	0	3	0	2
147	17	max	1204.096	1	.022	2	.23	1	0	1	0	1	0	12
148		min	-1190.759	3	-.15	3	-.112	3	0	15	0	3	0	2
149	18	max	1204.213	1	-.013	2	.23	1	0	1	0	1	0	12
150		min	-1190.672	3	-.177	3	-.112	3	0	15	0	3	0	2
151	19	max	1204.329	1	-.042	15	.23	1	0	1	0	1	0	12





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1190.584	3	-.203	3	-.112	3	0	15	0	3	0	2
153	M7	1	max	472.906	2	1.779	4	.02	1	0	2	0	2	2
154		min	-404.916	3	.418	15	-.002	10	0	3	0	3	0	12
155		2	max	472.837	2	1.602	4	.02	1	0	2	0	2	2
156		min	-404.968	3	.377	15	-.002	10	0	3	0	3	0	3
157		3	max	472.769	2	1.424	4	.02	1	0	2	0	2	2
158		min	-405.019	3	.335	15	-.002	10	0	3	0	3	0	3
159		4	max	472.7	2	1.247	4	.02	1	0	2	0	2	2
160		min	-405.071	3	.293	15	-.002	10	0	3	0	3	0	3
161		5	max	472.631	2	1.07	4	.02	1	0	2	0	2	15
162		min	-405.122	3	.252	15	-.002	10	0	3	0	3	0	3
163		6	max	472.563	2	.893	4	.02	1	0	2	0	2	15
164		min	-405.173	3	.21	15	-.002	10	0	3	0	3	0	4
165		7	max	472.494	2	.716	4	.02	1	0	2	0	2	15
166		min	-405.225	3	.169	15	-.002	10	0	3	0	3	0	4
167		8	max	472.426	2	.538	4	.02	1	0	2	0	2	15
168		min	-405.276	3	.127	15	-.002	10	0	3	0	3	-.001	4
169		9	max	472.357	2	.363	2	.02	1	0	2	0	2	15
170		min	-405.328	3	.085	15	-.002	10	0	3	0	3	-.001	4
171		10	max	472.288	2	.225	2	.02	1	0	2	0	2	15
172		min	-405.379	3	.017	12	-.002	10	0	3	0	3	-.001	4
173		11	max	472.22	2	.086	2	.02	1	0	2	0	2	15
174		min	-405.431	3	-.084	3	-.002	10	0	3	0	3	-.001	4
175		12	max	472.151	2	-.04	15	.02	1	0	2	0	2	15
176		min	-405.482	3	-.188	3	-.002	10	0	3	0	3	-.001	4
177		13	max	472.083	2	-.081	15	.02	1	0	2	0	2	15
178		min	-405.534	3	-.348	4	-.002	10	0	3	0	3	-.001	4
179		14	max	472.014	2	-.123	15	.02	1	0	2	0	2	15
180		min	-405.585	3	-.525	4	-.002	10	0	3	0	3	-.001	4
181		15	max	471.945	2	-.165	15	.02	1	0	2	0	2	15
182		min	-405.637	3	-.702	4	-.002	10	0	3	0	3	0	4
183		16	max	471.877	2	-.206	15	.02	1	0	2	0	2	15
184		min	-405.688	3	-.879	4	-.002	10	0	3	0	3	0	4
185		17	max	471.808	2	-.248	15	.02	1	0	2	0	2	15
186		min	-405.739	3	-1.056	4	-.002	10	0	3	0	3	0	4
187		18	max	471.74	2	-.29	15	.02	1	0	2	0	2	15
188		min	-405.791	3	-1.234	4	-.002	10	0	3	0	3	0	4
189		19	max	471.671	2	-.331	15	.02	1	0	2	0	2	1
190		min	-405.842	3	-1.411	4	-.002	10	0	3	0	3	0	1
191	M8	1	max	1443.6	1	0	1	.919	1	0	1	0	15	0
192		min	-112.344	3	0	1	-.288	3	0	1	0	1	0	1
193		2	max	1443.665	1	0	1	.919	1	0	1	0	1	0
194		min	-112.295	3	0	1	-.288	3	0	1	0	3	0	1
195		3	max	1443.73	1	0	1	.919	1	0	1	0	1	0
196		min	-112.247	3	0	1	-.288	3	0	1	0	3	0	1
197		4	max	1443.794	1	0	1	.919	1	0	1	0	1	0
198		min	-112.198	3	0	1	-.288	3	0	1	0	3	0	1
199		5	max	1443.859	1	0	1	.919	1	0	1	0	1	0
200		min	-112.15	3	0	1	-.288	3	0	1	0	3	0	1
201		6	max	1443.924	1	0	1	.919	1	0	1	0	1	0
202		min	-112.101	3	0	1	-.288	3	0	1	0	3	0	1
203		7	max	1443.988	1	0	1	.919	1	0	1	0	1	0
204		min	-112.053	3	0	1	-.288	3	0	1	0	3	0	1
205		8	max	1444.053	1	0	1	.919	1	0	1	0	1	0
206		min	-112.004	3	0	1	-.288	3	0	1	0	3	0	1
207		9	max	1444.118	1	0	1	.919	1	0	1	0	1	0
208		min	-111.956	3	0	1	-.288	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	1444.183	1	0	1	.919	1	0	1	0	1	0	1
210		min	-111.907	3	0	1	-.288	3	0	1	0	3	0	1
211	11	max	1444.247	1	0	1	.919	1	0	1	0	1	0	1
212		min	-111.859	3	0	1	-.288	3	0	1	0	3	0	1
213	12	max	1444.312	1	0	1	.919	1	0	1	0	1	0	1
214		min	-111.81	3	0	1	-.288	3	0	1	0	3	0	1
215	13	max	1444.377	1	0	1	.919	1	0	1	0	1	0	1
216		min	-111.762	3	0	1	-.288	3	0	1	0	3	0	1
217	14	max	1444.441	1	0	1	.919	1	0	1	.001	1	0	1
218		min	-111.713	3	0	1	-.288	3	0	1	0	3	0	1
219	15	max	1444.506	1	0	1	.919	1	0	1	.001	1	0	1
220		min	-111.665	3	0	1	-.288	3	0	1	0	3	0	1
221	16	max	1444.571	1	0	1	.919	1	0	1	.001	1	0	1
222		min	-111.616	3	0	1	-.288	3	0	1	0	3	0	1
223	17	max	1444.635	1	0	1	.919	1	0	1	.001	1	0	1
224		min	-111.567	3	0	1	-.288	3	0	1	0	3	0	1
225	18	max	1444.7	1	0	1	.919	1	0	1	.001	1	0	1
226		min	-111.519	3	0	1	-.288	3	0	1	0	3	0	1
227	19	max	1444.765	1	0	1	.919	1	0	1	.001	1	0	1
228		min	-111.47	3	0	1	-.288	3	0	1	0	3	0	1
229	M10	1	max	384.066	1	.633	4	-.009	15	.001	1	0	1	0
230		min	-350.321	3	.15	15	-.237	1	0	3	0	3	0	1
231	2	max	384.182	1	.587	4	-.009	15	.001	1	0	1	0	15
232		min	-350.234	3	.139	15	-.237	1	0	3	0	3	0	4
233	3	max	384.299	1	.541	4	-.009	15	.001	1	0	1	0	15
234		min	-350.147	3	.129	15	-.237	1	0	3	0	3	0	4
235	4	max	384.415	1	.496	4	-.009	15	.001	1	0	1	0	15
236		min	-350.059	3	.118	15	-.237	1	0	3	0	3	0	4
237	5	max	384.531	1	.45	4	-.009	15	.001	1	0	1	0	15
238		min	-349.972	3	.107	15	-.237	1	0	3	0	3	0	4
239	6	max	384.648	1	.404	4	-.009	15	.001	1	0	1	0	15
240		min	-349.885	3	.096	15	-.237	1	0	3	0	3	0	4
241	7	max	384.764	1	.359	4	-.009	15	.001	1	0	2	0	15
242		min	-349.797	3	.086	15	-.237	1	0	3	0	3	0	4
243	8	max	384.881	1	.313	4	-.009	15	.001	1	0	2	0	15
244		min	-349.71	3	.075	15	-.237	1	0	3	0	3	0	4
245	9	max	384.997	1	.267	4	-.009	15	.001	1	0	2	0	15
246		min	-349.623	3	.064	15	-.237	1	0	3	0	3	0	4
247	10	max	385.113	1	.222	4	-.009	15	.001	1	0	2	0	15
248		min	-349.535	3	.053	15	-.237	1	0	3	0	3	0	4
249	11	max	385.23	1	.176	4	-.009	15	.001	1	0	15	0	15
250		min	-349.448	3	.043	15	-.237	1	0	3	0	1	0	4
251	12	max	385.346	1	.13	4	-.009	15	.001	1	0	15	0	15
252		min	-349.361	3	.032	15	-.237	1	0	3	0	1	0	4
253	13	max	385.463	1	.085	4	-.009	15	.001	1	0	15	0	15
254		min	-349.274	3	.021	15	-.237	1	0	3	0	1	0	4
255	14	max	385.579	1	.052	10	-.009	15	.001	1	0	15	0	15
256		min	-349.186	3	-.007	1	-.237	1	0	3	0	1	0	4
257	15	max	385.695	1	.022	10	-.009	15	.001	1	0	15	0	15
258		min	-349.099	3	-.043	1	-.237	1	0	3	0	1	0	4
259	16	max	385.812	1	-.008	10	-.009	15	.001	1	0	15	0	15
260		min	-349.012	3	-.078	1	-.237	1	0	3	0	1	0	4
261	17	max	385.928	1	-.022	15	-.009	15	.001	1	0	15	0	15
262		min	-348.924	3	-.114	1	-.237	1	0	3	0	1	0	4
263	18	max	386.045	1	-.032	15	-.009	15	.001	1	0	15	0	15
264		min	-348.837	3	-.149	1	-.237	1	0	3	0	1	0	4
265	19	max	386.161	1	-.043	15	-.009	15	.001	1	0	15	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-348.75	3	-1.189	4	-.237	1	0	3	0	1	0	4
267			max	99.833	2	1.781	4	.969	1	.001	1	0	3	0	4
268			min	-131.399	3	.419	15	.021	12	0	15	-.003	1	0	15
269		2	max	99.765	2	1.604	4	.969	1	.001	1	0	3	0	2
270			min	-131.451	3	.377	15	.021	12	0	15	-.002	1	0	12
271		3	max	99.696	2	1.426	4	.969	1	.001	1	0	3	0	2
272			min	-131.502	3	.335	15	.021	12	0	15	-.002	1	0	3
273		4	max	99.627	2	1.249	4	.969	1	.001	1	0	3	0	15
274			min	-131.554	3	.294	15	.021	12	0	15	-.002	1	0	3
275		5	max	99.559	2	1.072	4	.969	1	.001	1	0	3	0	15
276		min	-131.605	3	.252	15	.021	12	0	15	-.002	1	0	4	
277	6	max	99.49	2	.895	4	.969	1	.001	1	0	3	0	15	
278		min	-131.657	3	.21	15	.021	12	0	15	-.002	1	0	4	
279	7	max	99.422	2	.718	4	.969	1	.001	1	0	3	0	15	
280		min	-131.708	3	.169	15	.021	12	0	15	-.001	1	0	4	
281	8	max	99.353	2	.54	4	.969	1	.001	1	0	3	0	15	
282		min	-131.759	3	.127	15	.021	12	0	15	-.001	1	-.001	4	
283	9	max	99.284	2	.363	4	.969	1	.001	1	0	3	0	15	
284		min	-131.811	3	.085	15	.021	12	0	15	0	1	-.001	4	
285	10	max	99.216	2	.186	4	.969	1	.001	1	0	3	0	15	
286		min	-131.862	3	.044	15	.021	12	0	15	0	1	-.001	4	
287	11	max	99.147	2	.044	2	.969	1	.001	1	0	3	0	15	
288		min	-131.914	3	-.039	3	.021	12	0	15	0	1	-.001	4	
289	12	max	99.078	2	-.039	15	.969	1	.001	1	0	3	0	15	
290		min	-131.965	3	-.168	4	.021	12	0	15	0	1	-.001	4	
291	13	max	99.01	2	-.081	15	.969	1	.001	1	0	3	0	15	
292		min	-132.017	3	-.346	4	.021	12	0	15	0	1	-.001	4	
293	14	max	98.941	2	-.123	15	.969	1	.001	1	0	3	0	15	
294		min	-132.068	3	-.523	4	.021	12	0	15	0	2	-.001	4	
295	15	max	98.873	2	-.164	15	.969	1	.001	1	0	1	0	15	
296		min	-132.12	3	-.7	4	.021	12	0	15	0	10	0	4	
297	16	max	98.804	2	-.206	15	.969	1	.001	1	0	1	0	15	
298		min	-132.171	3	-.877	4	.021	12	0	15	0	15	0	4	
299	17	max	98.735	2	-.248	15	.969	1	.001	1	0	1	0	15	
300		min	-132.223	3	-1.054	4	.021	12	0	15	0	15	0	4	
301	18	max	98.667	2	-.289	15	.969	1	.001	1	0	1	0	15	
302		min	-132.274	3	-1.232	4	.021	12	0	15	0	15	0	4	
303	19	max	98.598	2	-.331	15	.969	1	.001	1	.001	1	0	1	
304		min	-132.325	3	-1.409	4	.021	12	0	15	0	15	0	1	
305	M12	1	max	530.428	1	0	1	5.088	1	0	1	0	1	0	1
306			min	-24.181	3	0	1	.185	15	0	1	0	3	0	1
307		2	max	530.493	1	0	1	5.088	1	0	1	0	1	0	1
308			min	-24.133	3	0	1	.185	15	0	1	0	15	0	1
309		3	max	530.557	1	0	1	5.088	1	0	1	0	1	0	1
310			min	-24.084	3	0	1	.185	15	0	1	0	15	0	1
311		4	max	530.622	1	0	1	5.088	1	0	1	.001	1	0	1
312			min	-24.036	3	0	1	.185	15	0	1	0	15	0	1
313		5	max	530.687	1	0	1	5.088	1	0	1	.002	1	0	1
314			min	-23.987	3	0	1	.185	15	0	1	0	15	0	1
315	6	max	530.752	1	0	1	5.088	1	0	1	.002	1	0	1	
316		min	-23.938	3	0	1	.185	15	0	1	0	15	0	1	
317	7	max	530.816	1	0	1	5.088	1	0	1	.003	1	0	1	
318		min	-23.89	3	0	1	.185	15	0	1	0	15	0	1	
319	8	max	530.881	1	0	1	5.088	1	0	1	.003	1	0	1	
320		min	-23.841	3	0	1	.185	15	0	1	0	15	0	1	
321	9	max	530.946	1	0	1	5.088	1	0	1	.004	1	0	1	
322		min	-23.793	3	0	1	.185	15	0	1	0	15	0	1	



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	531.01	1	0	1	5.088	1	0	1	.004	1	0	1
324		min	-23.744	3	0	1	.185	15	0	1	0	15	0	1
325	11	max	531.075	1	0	1	5.088	1	0	1	.005	1	0	1
326		min	-23.696	3	0	1	.185	15	0	1	0	15	0	1
327	12	max	531.14	1	0	1	5.088	1	0	1	.005	1	0	1
328		min	-23.647	3	0	1	.185	15	0	1	0	15	0	1
329	13	max	531.204	1	0	1	5.088	1	0	1	.005	1	0	1
330		min	-23.599	3	0	1	.185	15	0	1	0	15	0	1
331	14	max	531.269	1	0	1	5.088	1	0	1	.006	1	0	1
332		min	-23.55	3	0	1	.185	15	0	1	0	15	0	1
333	15	max	531.334	1	0	1	5.088	1	0	1	.006	1	0	1
334		min	-23.502	3	0	1	.185	15	0	1	0	15	0	1
335	16	max	531.399	1	0	1	5.088	1	0	1	.007	1	0	1
336		min	-23.453	3	0	1	.185	15	0	1	0	15	0	1
337	17	max	531.463	1	0	1	5.088	1	0	1	.007	1	0	1
338		min	-23.405	3	0	1	.185	15	0	1	0	15	0	1
339	18	max	531.528	1	0	1	5.088	1	0	1	.008	1	0	1
340		min	-23.356	3	0	1	.185	15	0	1	0	15	0	1
341	19	max	531.593	1	0	1	5.088	1	0	1	.008	1	0	1
342		min	-23.308	3	0	1	.185	15	0	1	0	15	0	1
343	M1	1	max	161.948	1	342.178	3	-3.693	15	0	.197	1	.013	1
344		min	5.936	15	-363.767	1	-99.798	1	0	3	.007	15	-.01	3
345	2	max	162.066	1	341.988	3	-3.693	15	0	1	.176	1	.092	1
346		min	5.972	15	-364.02	1	-99.798	1	0	3	.006	15	-.084	3
347	3	max	111.573	1	7.892	9	-3.665	15	0	12	.152	1	.169	1
348		min	-1.269	10	-19.153	3	-99.666	1	0	1	.006	15	-.157	3
349	4	max	111.691	1	7.681	9	-3.665	15	0	12	.131	1	.169	1
350		min	-1.171	10	-19.343	3	-99.666	1	0	1	.005	15	-.153	3
351	5	max	111.809	1	7.47	9	-3.665	15	0	12	.109	1	.169	1
352		min	-1.072	10	-19.533	3	-99.666	1	0	1	.004	15	-.149	3
353	6	max	111.927	1	7.259	9	-3.665	15	0	12	.088	1	.17	1
354		min	-.974	10	-19.723	3	-99.666	1	0	1	.003	15	-.144	3
355	7	max	112.045	1	7.048	9	-3.665	15	0	12	.066	1	.17	1
356		min	-.876	10	-19.913	3	-99.666	1	0	1	.002	15	-.14	3
357	8	max	112.163	1	6.837	9	-3.665	15	0	12	.044	1	.17	1
358		min	-.777	10	-20.102	3	-99.666	1	0	1	.002	15	-.136	3
359	9	max	112.281	1	6.626	9	-3.665	15	0	12	.023	1	.171	1
360		min	-.679	10	-20.292	3	-99.666	1	0	1	0	15	-.131	3
361	10	max	112.399	1	6.415	9	-3.665	15	0	12	.001	1	.171	1
362		min	-.581	10	-20.482	3	-99.666	1	0	1	0	10	-.127	3
363	11	max	112.517	1	6.204	9	-3.665	15	0	12	0	12	.172	1
364		min	-.482	10	-20.672	3	-99.666	1	0	1	-.02	1	-.122	3
365	12	max	112.635	1	5.994	9	-3.665	15	0	12	-.001	12	.173	2
366		min	-.384	10	-20.862	3	-99.666	1	0	1	-.042	1	-.118	3
367	13	max	112.753	1	5.783	9	-3.665	15	0	12	-.002	12	.177	2
368		min	-.286	10	-21.051	3	-99.666	1	0	1	-.064	1	-.113	3
369	14	max	112.871	1	5.572	9	-3.665	15	0	12	-.003	15	.182	2
370		min	-.187	10	-21.241	3	-99.666	1	0	1	-.085	1	-.109	3
371	15	max	112.989	1	5.361	9	-3.665	15	0	12	-.004	15	.186	2
372		min	-.089	10	-21.473	2	-99.666	1	0	1	-.107	1	-.104	3
373	16	max	90.393	2	55.23	2	-3.696	15	0	1	-.005	15	.19	2
374		min	-19.584	3	-125.039	3	-100.389	1	0	12	-.13	1	-.099	3
375	17	max	90.511	2	54.977	2	-3.696	15	0	1	-.006	15	.19	1
376		min	-19.496	3	-125.229	3	-100.389	1	0	12	-.151	1	-.071	3
377	18	max	-5.942	15	411.258	1	-3.787	15	0	3	-.006	15	.103	1
378		min	-161.332	1	-147.822	3	-102.958	1	0	1	-.174	1	-.039	3
379	19	max	-5.907	15	411.005	1	-3.787	15	0	3	-.007	15	.014	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-161.214	1	-148.012	3	-102.958	1	0	1	-.196	1	-.007	3
381	M5	max	353.811	1	1131.257	3	-.128	10	0	1	.006	1	.02	3
382		min	11.621	12	-1203.993	1	-36.541	1	0	3	0	10	-.025	1
383		max	353.929	1	1131.067	3	-.128	10	0	1	0	2	.236	1
384		min	11.68	12	-1204.246	1	-36.541	1	0	3	-.003	3	-.225	3
385		max	189.376	1	8.278	9	3.604	3	0	3	0	10	.492	1
386		min	-20.852	10	-69.755	2	-.14	2	0	1	-.01	3	-.466	3
387		max	189.494	1	8.067	9	3.604	3	0	3	0	10	.498	1
388		min	-20.753	10	-70.008	2	-.14	2	0	1	-.009	3	-.452	3
389		max	189.612	1	7.856	9	3.604	3	0	3	0	10	.503	1
390		min	-20.655	10	-70.261	2	-.14	2	0	1	-.008	3	-.438	3
391		max	189.73	1	7.645	9	3.604	3	0	3	0	10	.509	1
392		min	-20.556	10	-70.515	2	-.14	2	0	1	-.007	3	-.424	3
393		max	189.848	1	7.434	9	3.604	3	0	3	0	10	.514	1
394		min	-20.458	10	-70.768	2	-.14	2	0	1	-.007	1	-.41	3
395		max	189.966	1	7.223	9	3.604	3	0	3	0	10	.52	1
396		min	-20.36	10	-71.021	2	-.14	2	0	1	-.006	1	-.396	3
397		max	190.084	1	7.012	9	3.604	3	0	3	0	10	.526	1
398		min	-20.261	10	-71.274	2	-.14	2	0	1	-.006	1	-.382	3
399		max	190.202	1	6.802	9	3.604	3	0	3	0	10	.532	1
400		min	-20.163	10	-71.527	2	-.14	2	0	1	-.005	1	-.368	3
401		max	190.32	1	6.591	9	3.604	3	0	3	0	10	.538	1
402		min	-20.065	10	-71.78	2	-.14	2	0	1	-.004	1	-.353	3
403		max	190.438	1	6.38	9	3.604	3	0	3	0	10	.543	1
404		min	-19.966	10	-72.033	2	-.14	2	0	1	-.004	1	-.339	3
405		max	190.556	1	6.169	9	3.604	3	0	3	0	10	.549	1
406		min	-19.868	10	-72.286	2	-.14	2	0	1	-.003	1	-.325	3
407		max	190.674	1	5.958	9	3.604	3	0	3	0	15	.556	2
408		min	-19.77	10	-72.539	2	-.14	2	0	1	-.003	1	-.311	3
409		max	190.792	1	5.747	9	3.604	3	0	3	0	15	.572	2
410		min	-19.671	10	-72.792	2	-.14	2	0	1	-.002	1	-.296	3
411		max	314.739	2	299.12	2	3.58	3	0	1	0	3	.584	2
412		min	-65.843	3	-383.728	3	-.155	2	0	15	-.002	1	-.279	3
413		max	314.857	2	298.867	2	3.58	3	0	1	0	3	.555	1
414		min	-65.754	3	-383.917	3	-.155	2	0	15	-.001	1	-.196	3
415		max	-12.238	12	1354.338	1	3.332	1	0	3	.002	3	.266	1
416		min	-354.912	1	-486.333	3	-.025	10	0	1	0	2	-.091	3
417		max	-12.179	12	1354.085	1	3.332	1	0	3	.002	3	.015	3
418		min	-354.794	1	-486.523	3	-.025	10	0	1	0	2	-.027	1
419	M9	max	161.175	1	342.162	3	136.26	1	0	3	-.007	15	.013	1
420		min	5.906	15	-363.743	1	5.2	15	0	1	-.197	1	-.01	3
421		max	161.293	1	341.972	3	136.26	1	0	3	-.005	12	.092	1
422		min	5.942	15	-363.996	1	5.2	15	0	1	-.167	1	-.084	3
423		max	111.474	1	7.863	9	93.642	1	0	1	0	3	.169	1
424		min	-.723	10	-19.095	3	2.03	12	0	15	-.136	1	-.157	3
425		max	111.592	1	7.652	9	93.642	1	0	1	.002	3	.169	1
426		min	-.625	10	-19.284	3	2.03	12	0	15	-.116	1	-.153	3
427		max	111.71	1	7.441	9	93.642	1	0	1	.002	3	.169	1
428		min	-.526	10	-19.474	3	2.03	12	0	15	-.096	1	-.149	3
429		max	111.828	1	7.23	9	93.642	1	0	1	.003	3	.17	1
430		min	-.428	10	-19.664	3	2.03	12	0	15	-.075	1	-.144	3
431		max	111.946	1	7.019	9	93.642	1	0	1	.003	3	.17	1
432		min	-.33	10	-19.854	3	2.03	12	0	15	-.055	1	-.14	3
433		max	112.064	1	6.809	9	93.642	1	0	1	.004	3	.17	1
434		min	-.231	10	-20.044	3	2.03	12	0	15	-.035	1	-.136	3
435		max	112.182	1	6.598	9	93.642	1	0	1	.005	3	.171	1
436		min	-.133	10	-20.233	3	2.03	12	0	15	-.015	1	-.131	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	112.3	1	6.387	9	93.642	1	0	1	.006	1	.171	1
438			min	-.035	10	-20.423	3	2.03	12	0	15	0	10	-.127	3
439		11	max	112.418	1	6.176	9	93.642	1	0	1	.026	1	.172	1
440			min	.064	10	-20.613	3	2.03	12	0	15	0	15	-.122	3
441		12	max	112.536	1	5.965	9	93.642	1	0	1	.046	1	.173	2
442			min	.162	10	-20.803	3	2.03	12	0	15	.002	15	-.118	3
443		13	max	112.654	1	5.754	9	93.642	1	0	1	.067	1	.177	2
444			min	.26	10	-20.993	3	2.03	12	0	15	.002	15	-.113	3
445		14	max	112.772	1	5.543	9	93.642	1	0	1	.087	1	.182	2
446			min	.359	10	-21.235	2	2.03	12	0	15	.003	15	-.109	3
447		15	max	112.89	1	5.332	9	93.642	1	0	1	.107	1	.186	2
448			min	.457	10	-21.488	2	2.03	12	0	15	.004	15	-.104	3
449		16	max	90.667	2	55.062	2	94.492	1	0	15	.129	1	.19	2
450			min	-19.599	3	-125.496	3	2.053	12	0	1	.005	15	-.099	3
451		17	max	90.785	2	54.809	2	94.492	1	0	15	.15	1	.19	1
452			min	-19.51	3	-125.685	3	2.053	12	0	1	.006	15	-.071	3
453		18	max	-5.932	15	411.258	1	99.647	1	0	1	.172	1	.103	1
454			min	-161.044	1	-147.82	3	2.349	12	0	3	.006	15	-.039	3
455		19	max	-5.896	15	411.004	1	99.647	1	0	1	.193	1	.014	1
456			min	-160.926	1	-148.01	3	2.349	12	0	3	.007	15	-.007	3
457	M13	1	max	136.604	1	363.063	1	-5.906	15	.013	1	.197	1	0	1
458			min	5.2	15	-342.143	3	-161.153	1	-.01	3	.007	15	0	3
459		2	max	136.604	1	256.121	1	-4.53	15	.013	1	.063	1	.275	3
460			min	5.2	15	-241.292	3	-123.546	1	-.01	3	.002	15	-.292	1
461		3	max	136.604	1	149.179	1	-3.155	15	.013	1	.001	3	.456	3
462			min	5.2	15	-140.441	3	-85.938	1	-.01	3	-.036	1	-.484	1
463		4	max	136.604	1	42.237	1	-1.779	15	.013	1	-.002	12	.541	3
464			min	5.2	15	-39.589	3	-48.331	1	-.01	3	-.1	1	-.574	1
465		5	max	136.604	1	61.262	3	-.404	15	.013	1	-.003	12	.531	3
466			min	5.2	15	-64.705	1	-10.723	1	-.01	3	-.128	1	-.564	1
467		6	max	136.604	1	162.113	3	26.884	1	.013	1	-.004	12	.425	3
468			min	5.2	15	-171.647	1	.5	12	-.01	3	-.12	1	-.452	1
469		7	max	136.604	1	262.965	3	64.492	1	.013	1	-.002	12	.224	3
470			min	5.2	15	-278.589	1	1.842	12	-.01	3	-.077	1	-.239	1
471		8	max	136.604	1	363.816	3	102.099	1	.013	1	.002	1	.074	1
472			min	5.2	15	-385.531	1	3.184	12	-.01	3	0	3	-.072	3
473		9	max	136.604	1	464.667	3	139.707	1	.013	1	.116	1	.489	1
474			min	5.2	15	-492.474	1	4.527	12	-.01	3	.004	12	-.463	3
475		10	max	136.604	1	565.518	3	177.314	1	.011	2	.266	1	1.005	1
476			min	5.2	15	-599.416	1	5.869	12	-.013	1	.008	12	-.949	3
477		11	max	100.191	1	492.473	1	-4.407	12	.01	3	.11	1	.489	1
478			min	3.694	15	-464.667	3	-138.928	1	-.013	1	.001	12	-.463	3
479		12	max	100.191	1	385.531	1	-3.065	12	.01	3	0	10	.074	1
480			min	3.694	15	-363.816	3	-101.321	1	-.013	1	-.003	3	-.072	3
481		13	max	100.191	1	278.589	1	-1.723	12	.01	3	-.003	15	.224	3
482			min	3.694	15	-262.964	3	-63.713	1	-.013	1	-.081	1	-.239	1
483		14	max	100.191	1	171.647	1	-.38	12	.01	3	-.005	15	.425	3
484			min	3.694	15	-162.113	3	-26.106	1	-.013	1	-.123	1	-.452	1
485		15	max	100.191	1	64.705	1	11.502	1	.01	3	-.005	15	.531	3
486			min	3.694	15	-61.262	3	.434	15	-.013	1	-.13	1	-.564	1
487		16	max	100.191	1	39.589	3	49.109	1	.01	3	-.003	12	.541	3
488			min	3.694	15	-42.237	1	1.809	15	-.013	1	-.102	1	-.574	1
489		17	max	100.191	1	140.441	3	86.717	1	.01	3	0	12	.456	3
490			min	3.694	15	-149.179	1	3.185	15	-.013	1	-.038	1	-.484	1
491		18	max	100.191	1	241.292	3	124.324	1	.01	3	.062	1	.275	3
492			min	3.694	15	-256.121	1	4.561	15	-.013	1	.002	15	-.292	1
493		19	max	100.191	1	342.143	3	161.932	1	.01	3	.197	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	3.694	15	-363.064	1	5.936	15	-.013	1	.007	15	0	3
495		max	-2.348	12	411.732	1	-5.896	15	.007	3	.193	1	0	1
496		min	-99.216	1	-148.036	3	-160.943	1	-.014	1	.007	15	0	3
497		2 max	-2.348	12	290.449	1	-4.521	15	.007	3	.059	1	.119	3
498		min	-99.216	1	-104.55	3	-123.335	1	-.014	1	.002	15	-.332	1
499		3 max	-2.348	12	169.166	1	-3.145	15	.007	3	-.001	12	.197	3
500		min	-99.216	1	-61.063	3	-85.728	1	-.014	1	-.04	1	-.549	1
501		4 max	-2.348	12	47.882	1	-1.77	15	.007	3	-.004	15	.235	3
502		min	-99.216	1	-17.577	3	-48.12	1	-.014	1	-.103	1	-.651	1
503		5 max	-2.348	12	25.91	3	-.394	15	.007	3	-.005	15	.231	3
504		min	-99.216	1	-73.401	1	-10.513	1	-.014	1	-.131	1	-.639	1
505		6 max	-2.348	12	69.396	3	27.095	1	.007	3	-.005	15	.186	3
506		min	-99.216	1	-194.684	1	.65	12	-.014	1	-.123	1	-.512	1
507		7 max	-2.348	12	112.883	3	64.702	1	.007	3	-.003	15	.1	3
508		min	-99.216	1	-315.968	1	1.992	12	-.014	1	-.08	1	-.271	1
509		8 max	-2.348	12	156.369	3	102.31	1	.007	3	.001	2	.084	1
510		min	-99.216	1	-437.251	1	3.334	12	-.014	1	-.002	3	-.028	3
511		9 max	-2.348	12	199.856	3	139.917	1	.007	3	.114	1	.555	1
512		min	-99.216	1	-558.534	1	4.676	12	-.014	1	.002	12	-.196	3
513		10 max	-3.786	15	-16.537	15	177.525	1	0	15	.264	1	1.139	1
514		min	-102.583	1	-679.818	1	-9.226	3	-.014	1	.009	12	-.405	3
515		11 max	-3.786	15	558.534	1	-4.818	12	.014	1	.114	1	.555	1
516		min	-102.583	1	-199.856	3	-139.628	1	-.007	3	.004	12	-.196	3
517		12 max	-3.786	15	437.251	1	-3.476	12	.014	1	0	2	.084	1
518		min	-102.583	1	-156.369	3	-102.021	1	-.007	3	0	3	-.028	3
519		13 max	-3.786	15	315.968	1	-2.134	12	.014	1	-.003	12	.1	3
520		min	-102.583	1	-112.883	3	-64.413	1	-.007	3	-.078	1	-.271	1
521		14 max	-3.786	15	194.684	1	-.792	12	.014	1	-.004	12	.186	3
522		min	-102.583	1	-69.396	3	-26.806	1	-.007	3	-.121	1	-.512	1
523		15 max	-3.786	15	73.401	1	10.801	1	.014	1	-.004	12	.231	3
524		min	-102.583	1	-25.91	3	.404	15	-.007	3	-.129	1	-.639	1
525		16 max	-3.786	15	17.577	3	48.409	1	.014	1	-.003	12	.235	3
526		min	-102.583	1	-47.882	1	1.78	15	-.007	3	-.101	1	-.651	1
527		17 max	-3.786	15	61.063	3	86.016	1	.014	1	0	12	.197	3
528		min	-102.583	1	-169.166	1	3.155	15	-.007	3	-.038	1	-.549	1
529		18 max	-3.786	15	104.55	3	123.624	1	.014	1	.061	1	.119	3
530		min	-102.583	1	-290.449	1	4.531	15	-.007	3	.002	15	-.332	1
531		19 max	-3.786	15	148.036	3	161.231	1	.014	1	.196	1	0	1
532		min	-102.583	1	-411.732	1	5.907	15	-.007	3	.007	15	0	3
533	M15	1 max	0	10	2.961	4	.024	3	0	1	0	1	0	1
534		min	-40.623	1	0	10	-.027	1	0	3	0	3	0	1
535		2 max	0	10	2.632	4	.024	3	0	1	0	1	0	10
536		min	-40.71	1	0	10	-.027	1	0	3	0	3	-.001	4
537		3 max	0	10	2.303	4	.024	3	0	1	0	1	0	10
538		min	-40.797	1	0	10	-.027	1	0	3	0	3	-.003	4
539		4 max	0	10	1.974	4	.024	3	0	1	0	1	0	10
540		min	-40.884	1	0	10	-.027	1	0	3	0	3	-.004	4
541		5 max	0	10	1.645	4	.024	3	0	1	0	1	0	10
542		min	-40.971	1	0	10	-.027	1	0	3	0	3	-.005	4
543		6 max	0	10	1.316	4	.024	3	0	1	0	1	0	10
544		min	-41.058	1	0	10	-.027	1	0	3	0	3	-.005	4
545		7 max	0	10	.987	4	.024	3	0	1	0	3	0	10
546		min	-41.145	1	0	10	-.027	1	0	3	0	2	-.006	4
547		8 max	0	10	.658	4	.024	3	0	1	0	3	0	10
548		min	-41.232	1	0	10	-.027	1	0	3	0	1	-.006	4
549		9 max	0	10	.329	4	.024	3	0	1	0	3	0	10
550		min	-41.319	1	0	10	-.027	1	0	3	0	1	-.007	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	10	0	1	.024	3	0	1	0	3	0	10
552			min	-41.406	1	0	1	-.027	1	0	3	0	1	-.007	4
553		11	max	0	10	0	10	.024	3	0	1	0	3	0	10
554			min	-41.493	1	-.329	4	-.027	1	0	3	0	1	-.007	4
555		12	max	0	10	0	10	.024	3	0	1	0	3	0	10
556			min	-41.579	1	-.658	4	-.027	1	0	3	0	1	-.006	4
557		13	max	0	10	0	10	.024	3	0	1	0	3	0	10
558			min	-41.666	1	-.987	4	-.027	1	0	3	0	1	-.006	4
559		14	max	0	10	0	10	.024	3	0	1	0	3	0	10
560			min	-41.753	1	-1.316	4	-.027	1	0	3	0	1	-.005	4
561		15	max	0	10	0	10	.024	3	0	1	0	3	0	10
562			min	-41.84	1	-1.645	4	-.027	1	0	3	0	1	-.005	4
563		16	max	0	10	0	10	.024	3	0	1	0	3	0	10
564			min	-41.927	1	-1.974	4	-.027	1	0	3	0	1	-.004	4
565		17	max	0	10	0	10	.024	3	0	1	0	3	0	10
566			min	-42.014	1	-2.303	4	-.027	1	0	3	0	1	-.003	4
567		18	max	0	10	0	10	.024	3	0	1	0	3	0	10
568			min	-42.101	1	-2.632	4	-.027	1	0	3	0	1	-.001	4
569		19	max	0	10	0	10	.024	3	0	1	0	3	0	1
570			min	-42.188	1	-2.961	4	-.027	1	0	3	0	1	0	1
571	M16A	1	max	-1.024	10	2.961	4	.02	1	0	3	0	3	0	1
572			min	-46.249	1	.696	15	-.01	3	0	1	0	1	0	1
573		2	max	-.952	10	2.632	4	.02	1	0	3	0	3	0	15
574			min	-46.162	1	.619	15	-.01	3	0	1	0	1	-.001	4
575		3	max	-.88	10	2.303	4	.02	1	0	3	0	3	0	15
576			min	-46.075	1	.541	15	-.01	3	0	1	0	1	-.003	4
577		4	max	-.807	10	1.974	4	.02	1	0	3	0	3	0	15
578			min	-45.988	1	.464	15	-.01	3	0	1	0	1	-.004	4
579		5	max	-.735	10	1.645	4	.02	1	0	3	0	3	-.001	15
580			min	-45.901	1	.387	15	-.01	3	0	1	0	1	-.005	4
581		6	max	-.662	10	1.316	4	.02	1	0	3	0	3	-.001	15
582			min	-45.814	1	.309	15	-.01	3	0	1	0	1	-.005	4
583		7	max	-.59	10	.987	4	.02	1	0	3	0	3	-.001	15
584			min	-45.727	1	.232	15	-.01	3	0	1	0	1	-.006	4
585		8	max	-.517	10	.658	4	.02	1	0	3	0	3	-.001	15
586			min	-45.641	1	.155	15	-.01	3	0	1	0	1	-.006	4
587		9	max	-.445	10	.329	4	.02	1	0	3	0	3	-.002	15
588			min	-45.554	1	.077	15	-.01	3	0	1	0	1	-.007	4
589		10	max	-.372	10	0	1	.02	1	0	3	0	3	-.002	15
590			min	-45.467	1	0	1	-.01	3	0	1	0	1	-.007	4
591		11	max	-.3	10	-.077	15	.02	1	0	3	0	3	-.002	15
592			min	-45.38	1	-.329	4	-.01	3	0	1	0	1	-.007	4
593		12	max	-.228	10	-.155	15	.02	1	0	3	0	3	-.001	15
594			min	-45.293	1	-.658	4	-.01	3	0	1	0	1	-.006	4
595		13	max	-.155	10	-.232	15	.02	1	0	3	0	1	-.001	15
596			min	-45.206	1	-.987	4	-.01	3	0	1	0	3	-.006	4
597		14	max	-.083	10	-.309	15	.02	1	0	3	0	1	-.001	15
598			min	-45.119	1	-1.316	4	-.01	3	0	1	0	3	-.005	4
599		15	max	-.01	10	-.387	15	.02	1	0	3	0	1	-.001	15
600			min	-45.032	1	-1.645	4	-.01	3	0	1	0	3	-.005	4
601		16	max	.062	10	-.464	15	.02	1	0	3	0	1	0	15
602			min	-44.945	1	-1.974	4	-.01	3	0	1	0	3	-.004	4
603		17	max	.135	10	-.541	15	.02	1	0	3	0	1	0	15
604			min	-44.858	1	-2.303	4	-.01	3	0	1	0	3	-.003	4
605		18	max	.207	10	-.619	15	.02	1	0	3	0	1	0	15
606			min	-44.771	1	-2.632	4	-.01	3	0	1	0	3	-.001	4
607		19	max	.279	10	-.696	15	.02	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-44.684	1	-2.961	4	-.01	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.009	2	.018	1	-5.805e-5	15	NC	3	NC	3	
2			min	-.003	3	-.008	3	0	3	-1.579e-3	1	4166.282	2	1970.15	1	
3			2	max	.003	1	.008	2	.017	1	-5.557e-5	15	NC	3	NC	3
4				min	-.003	3	-.008	3	0	3	-1.512e-3	1	4526.106	2	2126.925	1
5			3	max	.003	1	.007	2	.016	1	-5.309e-5	15	NC	3	NC	3
6				min	-.003	3	-.007	3	0	3	-1.445e-3	1	4950.335	2	2311.75	1
7			4	max	.003	1	.007	2	.014	1	-5.061e-5	15	NC	3	NC	3
8				min	-.003	3	-.007	3	0	3	-1.378e-3	1	5453.797	2	2531.371	1
9			5	max	.003	1	.006	2	.013	1	-4.814e-5	15	NC	3	NC	3
10				min	-.003	3	-.007	3	0	3	-1.311e-3	1	6056.053	2	2794.784	1
11		6	max	.002	1	.005	2	.012	1	-4.566e-5	15	NC	1	NC	3	
12			min	-.002	3	-.006	3	0	3	-1.244e-3	1	6783.314	2	3114.213	1	
13		7	max	.002	1	.005	2	.01	1	-4.318e-5	15	NC	1	NC	3	
14			min	-.002	3	-.006	3	0	3	-1.177e-3	1	7671.363	2	3506.601	1	
15		8	max	.002	1	.004	2	.009	1	-4.071e-5	15	NC	1	NC	3	
16			min	-.002	3	-.006	3	0	3	-1.11e-3	1	8770.114	2	3996.009	1	
17		9	max	.002	1	.004	2	.008	1	-3.823e-5	15	NC	1	NC	2	
18			min	-.002	3	-.005	3	0	3	-1.043e-3	1	NC	1	4617.561	1	
19		10	max	.002	1	.003	2	.007	1	-3.575e-5	15	NC	1	NC	2	
20			min	-.002	3	-.005	3	0	3	-9.763e-4	1	NC	1	5424.246	1	
21		11	max	.001	1	.003	2	.006	1	-3.327e-5	15	NC	1	NC	2	
22			min	-.001	3	-.004	3	0	3	-9.094e-4	1	NC	1	6499.22	1	
23		12	max	.001	1	.002	2	.005	1	-3.08e-5	15	NC	1	NC	2	
24			min	-.001	3	-.004	3	0	3	-8.425e-4	1	NC	1	7979.413	1	
25		13	max	.001	1	.002	2	.004	1	-2.832e-5	15	NC	1	NC	1	
26			min	-.001	3	-.003	3	0	3	-7.755e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.003	1	-2.584e-5	15	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-7.086e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-2.336e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-6.417e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-2.089e-5	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	12	-5.748e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.841e-5	15	NC	1	NC	1	
34			min	0	3	-.001	3	0	12	-5.078e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.593e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-4.409e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-1.165e-5	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.74e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.741e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	5.531e-6	12	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	2.173e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	7.87e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	2.605e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	-.001	1	9.474e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	3.037e-4	1	NC	1	NC	1
46				min	0	2	-.002	3	-.002	1	1.108e-5	15	NC	1	NC	1
47			5	max	0	3	0	2	0	12	3.469e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	-.002	1	1.268e-5	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	3.901e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.429e-5	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	4.333e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	-.001	1	1.589e-5	15	NC	1	NC	1
53	8	max	0	3	.001	2	0	3	4.765e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	1.75e-5	15	NC	1	NC	1
55	9	max	0	3	.002	2	0	3	5.196e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	2	1.91e-5	15	NC	1	NC	1
57	10	max	0	3	.002	2	0	1	5.628e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	15	2.07e-5	15	NC	1	NC	1
59	11	max	0	3	.003	2	.002	1	6.06e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	15	2.231e-5	15	NC	1	NC	1
61	12	max	0	3	.003	2	.003	1	6.492e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	2.391e-5	15	NC	1	NC	1
63	13	max	0	3	.004	2	.004	1	6.924e-4	1	NC	1	NC	1
64		min	0	2	-.008	3	0	15	2.552e-5	15	NC	1	NC	1
65	14	max	.001	3	.005	2	.005	1	7.356e-4	1	NC	1	NC	1
66		min	0	2	-.008	3	0	15	2.712e-5	15	9523.667	2	NC	1
67	15	max	.001	3	.006	2	.006	1	7.788e-4	1	NC	1	NC	2
68		min	0	2	-.008	3	0	15	2.873e-5	15	8039.685	2	8103.79	1
69	16	max	.001	3	.007	2	.007	1	8.22e-4	1	NC	3	NC	2
70		min	0	2	-.008	3	0	15	3.033e-5	15	6883.618	2	6786.769	1
71	17	max	.001	3	.008	2	.008	1	8.651e-4	1	NC	3	NC	2
72		min	-.001	2	-.008	3	0	15	3.193e-5	15	5974.111	2	5842.406	1
73	18	max	.001	3	.009	2	.009	1	9.083e-4	1	NC	3	NC	2
74		min	-.001	2	-.008	3	0	15	3.354e-5	15	5252.472	2	5142.789	1
75	19	max	.001	3	.01	2	.01	1	9.515e-4	1	NC	3	NC	2
76		min	-.001	2	-.008	3	0	15	3.514e-5	15	4676.077	2	4612.613	1
77	M4	1	max	.003	1	.01	2	15	-4.558e-5	15	NC	1	NC	3
78		min	0	3	-.008	3	-.008	1	-1.252e-3	1	NC	1	2573.871	1
79	2	max	.002	1	.01	2	0	15	-4.558e-5	15	NC	1	NC	3
80		min	0	3	-.008	3	-.007	1	-1.252e-3	1	NC	1	2807.367	1
81	3	max	.002	1	.009	2	0	15	-4.558e-5	15	NC	1	NC	3
82		min	0	3	-.007	3	-.006	1	-1.252e-3	1	NC	1	3085.288	1
83	4	max	.002	1	.008	2	0	15	-4.558e-5	15	NC	1	NC	3
84		min	0	3	-.007	3	-.006	1	-1.252e-3	1	NC	1	3419.347	1
85	5	max	.002	1	.008	2	0	15	-4.558e-5	15	NC	1	NC	2
86		min	0	3	-.006	3	-.005	1	-1.252e-3	1	NC	1	3825.501	1
87	6	max	.002	1	.007	2	0	15	-4.558e-5	15	NC	1	NC	2
88		min	0	3	-.006	3	-.004	1	-1.252e-3	1	NC	1	4325.945	1
89	7	max	.002	1	.007	2	0	15	-4.558e-5	15	NC	1	NC	2
90		min	0	3	-.005	3	-.004	1	-1.252e-3	1	NC	1	4952.294	1
91	8	max	.002	1	.006	2	0	15	-4.558e-5	15	NC	1	NC	2
92		min	0	3	-.005	3	-.003	1	-1.252e-3	1	NC	1	5750.842	1
93	9	max	.001	1	.006	2	0	15	-4.558e-5	15	NC	1	NC	2
94		min	0	3	-.004	3	-.003	1	-1.252e-3	1	NC	1	6791.624	1
95	10	max	.001	1	.005	2	0	15	-4.558e-5	15	NC	1	NC	2
96		min	0	3	-.004	3	-.002	1	-1.252e-3	1	NC	1	8184.744	1
97	11	max	.001	1	.004	2	0	15	-4.558e-5	15	NC	1	NC	1
98		min	0	3	-.004	3	-.002	1	-1.252e-3	1	NC	1	NC	1
99	12	max	0	1	.004	2	0	15	-4.558e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	-.001	1	-1.252e-3	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-4.558e-5	15	NC	1	NC	1
102		min	0	3	-.003	3	-.001	1	-1.252e-3	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-4.558e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-1.252e-3	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-4.558e-5	15	NC	1	NC	1
106		min	0	3	-.002	3	0	1	-1.252e-3	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-4.558e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-1.252e-3	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-4.558e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-1.252e-3	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-4.558e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-1.252e-3	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-4.558e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.252e-3	1	NC	1	NC	1
115	M6	1	max	.011	1	.029	2	.005	1	3.018e-4	1	NC	3	NC	2
116			min	-.011	3	-.023	3	-.003	3	3.817e-6	10	1255.412	2	7129.532	1
117		2	max	.01	1	.027	2	.005	1	2.834e-4	1	NC	3	NC	2
118			min	-.01	3	-.022	3	-.002	3	2.792e-6	10	1341.381	2	7757.913	1
119		3	max	.01	1	.025	2	.004	1	2.649e-4	1	NC	3	NC	2
120			min	-.009	3	-.021	3	-.002	3	1.766e-6	10	1439.644	2	8503.439	1
121		4	max	.009	1	.023	2	.004	1	2.465e-4	1	NC	3	NC	2
122			min	-.009	3	-.02	3	-.002	3	7.409e-7	10	1552.658	2	9396.228	1
123		5	max	.008	1	.022	2	.003	1	2.28e-4	1	NC	3	NC	1
124			min	-.008	3	-.019	3	-.002	3	-2.845e-7	10	1683.586	2	NC	1
125		6	max	.008	1	.02	2	.003	1	2.095e-4	1	NC	3	NC	1
126			min	-.008	3	-.017	3	-.002	3	-1.31e-6	10	1836.573	2	NC	1
127		7	max	.007	1	.018	2	.003	1	1.911e-4	1	NC	3	NC	1
128			min	-.007	3	-.016	3	-.002	3	-2.335e-6	10	2017.145	2	NC	1
129		8	max	.007	1	.016	2	.002	1	1.767e-4	3	NC	3	NC	1
130			min	-.007	3	-.015	3	-.001	3	-3.361e-6	10	2232.844	2	NC	1
131		9	max	.006	1	.015	2	.002	1	1.699e-4	3	NC	3	NC	1
132			min	-.006	3	-.014	3	-.001	3	-4.386e-6	10	2494.228	2	NC	1
133		10	max	.005	1	.013	2	.002	1	1.631e-4	3	NC	3	NC	1
134			min	-.005	3	-.012	3	-.001	3	-5.412e-6	10	2816.552	2	NC	1
135		11	max	.005	1	.011	2	.001	1	1.562e-4	3	NC	3	NC	1
136			min	-.005	3	-.011	3	0	3	-6.437e-6	10	3222.693	2	NC	1
137		12	max	.004	1	.01	2	.001	1	1.494e-4	3	NC	3	NC	1
138			min	-.004	3	-.01	3	0	3	-1.162e-5	2	3748.602	2	NC	1
139		13	max	.004	1	.008	2	0	1	1.425e-4	3	NC	3	NC	1
140			min	-.004	3	-.008	3	0	3	-1.697e-5	2	4454.186	2	NC	1
141		14	max	.003	1	.007	2	0	1	1.357e-4	3	NC	3	NC	1
142			min	-.003	3	-.007	3	0	3	-2.231e-5	2	5447.269	2	NC	1
143		15	max	.002	1	.005	2	0	1	1.288e-4	3	NC	3	NC	1
144			min	-.002	3	-.006	3	0	3	-2.766e-5	2	6943.47	2	NC	1
145		16	max	.002	1	.004	2	0	1	1.22e-4	3	NC	1	NC	1
146			min	-.002	3	-.004	3	0	3	-3.301e-5	2	9445.869	2	NC	1
147		17	max	.001	1	.003	2	0	1	1.151e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-3.836e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	2	1.083e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-4.371e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.015e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.906e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.254e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-4.7e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.917e-5	2	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-3.475e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.745e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	2	-2.251e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.938e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	2	-1.026e-5	3	NC	1	NC	1
161		5	max	.001	3	.006	2	0	3	2.131e-5	1	NC	3	NC	1
162			min	-.001	2	-.007	3	0	2	6.368e-7	15	8068.848	2	NC	1
163		6	max	.001	3	.007	2	0	3	2.323e-5	1	NC	3	NC	1
164			min	-.001	2	-.009	3	0	2	7.811e-7	15	6471.847	2	NC	1
165		7	max	.002	3	.009	2	.001	3	2.647e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.01	3	0	2	9.254e-7	15	5381.279	2	NC	1
167		8	max	.002	3	.01	2	.001	3	3.872e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-1.041e-6	2	4582.779	2	NC	1
169		9	max	.002	3	.012	2	.001	3	5.096e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	1	-4.41e-6	2	3969.521	2	NC	1
171		10	max	.002	3	.013	2	.001	3	6.321e-5	3	NC	3	NC	1
172			min	-.003	2	-.015	3	-.001	1	-7.778e-6	2	3482.301	2	NC	1
173		11	max	.003	3	.015	2	.001	3	7.545e-5	3	NC	3	NC	1
174			min	-.003	2	-.016	3	-.001	1	-1.115e-5	2	3085.594	2	NC	1
175		12	max	.003	3	.017	2	.002	3	8.77e-5	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-1.451e-5	2	2756.692	2	NC	1
177		13	max	.003	3	.019	2	.002	3	9.994e-5	3	NC	3	NC	1
178			min	-.004	2	-.018	3	-.002	1	-1.788e-5	2	2480.311	2	NC	1
179		14	max	.003	3	.021	2	.002	3	1.122e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	-.002	1	-2.125e-5	2	2245.703	2	NC	1
181		15	max	.004	3	.023	2	.002	3	1.244e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	-.002	1	-2.462e-5	2	2045.028	2	NC	1
183		16	max	.004	3	.025	2	.002	3	1.367e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	-.002	1	-2.799e-5	2	1872.39	2	NC	1
185		17	max	.004	3	.027	2	.001	3	1.489e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	-.002	1	-3.135e-5	2	1723.244	2	NC	1
187		18	max	.004	3	.029	2	.001	3	1.612e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	-.002	1	-3.472e-5	2	1594.014	2	NC	1
189		19	max	.005	3	.031	2	.001	3	1.734e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	-.003	1	-3.809e-5	2	1481.845	2	NC	1
191	M8	1	max	.007	1	.033	2	.003	1	-4.523e-6	10	NC	1	NC	2
192			min	0	3	-.023	3	0	3	-2.08e-4	1	NC	1	6665.917	1
193		2	max	.006	1	.031	2	.003	1	-4.523e-6	10	NC	1	NC	2
194			min	0	3	-.022	3	0	3	-2.08e-4	1	NC	1	7267.625	1
195		3	max	.006	1	.029	2	.002	1	-4.523e-6	10	NC	1	NC	2
196			min	0	3	-.021	3	0	3	-2.08e-4	1	NC	1	7983.961	1
197		4	max	.006	1	.027	2	.002	1	-4.523e-6	10	NC	1	NC	2
198			min	0	3	-.02	3	0	3	-2.08e-4	1	NC	1	8845.119	1
199		5	max	.005	1	.026	2	.002	1	-4.523e-6	10	NC	1	NC	2
200			min	0	3	-.018	3	0	3	-2.08e-4	1	NC	1	9892.235	1
201		6	max	.005	1	.024	2	.002	1	-4.523e-6	10	NC	1	NC	1
202			min	0	3	-.017	3	0	3	-2.08e-4	1	NC	1	NC	1
203		7	max	.005	1	.022	2	.002	1	-4.523e-6	10	NC	1	NC	1
204			min	0	3	-.016	3	0	3	-2.08e-4	1	NC	1	NC	1
205		8	max	.004	1	.02	2	.001	1	-4.523e-6	10	NC	1	NC	1
206			min	0	3	-.014	3	0	3	-2.08e-4	1	NC	1	NC	1
207		9	max	.004	1	.018	2	.001	1	-4.523e-6	10	NC	1	NC	1
208			min	0	3	-.013	3	0	3	-2.08e-4	1	NC	1	NC	1
209		10	max	.003	1	.016	2	0	1	-4.523e-6	10	NC	1	NC	1
210			min	0	3	-.012	3	0	3	-2.08e-4	1	NC	1	NC	1
211		11	max	.003	1	.015	2	0	1	-4.523e-6	10	NC	1	NC	1
212			min	0	3	-.01	3	0	3	-2.08e-4	1	NC	1	NC	1
213		12	max	.003	1	.013	2	0	1	-4.523e-6	10	NC	1	NC	1
214			min	0	3	-.009	3	0	3	-2.08e-4	1	NC	1	NC	1
215		13	max	.002	1	.011	2	0	1	-4.523e-6	10	NC	1	NC	1
216			min	0	3	-.008	3	0	3	-2.08e-4	1	NC	1	NC	1
217		14	max	.002	1	.009	2	0	1	-4.523e-6	10	NC	1	NC	1
218			min	0	3	-.007	3	0	3	-2.08e-4	1	NC	1	NC	1
219		15	max	.002	1	.007	2	0	1	-4.523e-6	10	NC	1	NC	1
220			min	0	3	-.005	3	0	3	-2.08e-4	1	NC	1	NC	1
221		16	max	.001	1	.005	2	0	1	-4.523e-6	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-2.08e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	1	-4.523e-6	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.08e-4	1	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-4.523e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-2.08e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-4.523e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.08e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.009	2	0	3	1.27e-3	1	NC	3	NC	1
230			min	-.003	3	-.008	3	-.003	1	-1.978e-4	3	4168.396	2	NC	1
231		2	max	.003	1	.008	2	0	3	1.203e-3	1	NC	3	NC	1
232			min	-.003	3	-.008	3	-.002	1	-1.922e-4	3	4517.821	2	NC	1
233		3	max	.003	1	.007	2	0	3	1.136e-3	1	NC	3	NC	1
234			min	-.003	3	-.007	3	-.002	1	-1.866e-4	3	4927.834	2	NC	1
235		4	max	.003	1	.007	2	0	3	1.069e-3	1	NC	3	NC	1
236			min	-.003	3	-.007	3	-.002	1	-1.811e-4	3	5411.877	2	NC	1
237		5	max	.003	1	.006	2	0	3	1.002e-3	1	NC	3	NC	1
238			min	-.002	3	-.007	3	-.002	1	-1.755e-4	3	5987.549	2	NC	1
239		6	max	.002	1	.005	2	0	3	9.347e-4	1	NC	1	NC	1
240			min	-.002	3	-.006	3	-.002	1	-1.699e-4	3	6678.24	2	NC	1
241		7	max	.002	1	.005	2	0	3	8.676e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	-.002	1	-1.644e-4	3	7515.611	2	NC	1
243		8	max	.002	1	.004	2	0	3	8.006e-4	1	NC	1	NC	1
244			min	-.002	3	-.006	3	-.001	1	-1.588e-4	3	8543.428	2	NC	1
245		9	max	.002	1	.004	2	0	3	7.336e-4	1	NC	1	NC	1
246			min	-.002	3	-.005	3	-.001	1	-1.532e-4	3	9823.72	2	NC	1
247		10	max	.002	1	.003	2	0	3	6.665e-4	1	NC	1	NC	1
248			min	-.002	3	-.005	3	-.001	1	-1.477e-4	3	NC	1	NC	1
249		11	max	.002	1	.003	2	0	3	5.995e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.421e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	5.325e-4	1	NC	1	NC	1
252			min	-.001	3	-.004	3	0	1	-1.365e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	4.655e-4	1	NC	1	NC	1
254			min	-.001	3	-.004	3	0	1	-1.31e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.984e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-1.254e-4	3	NC	1	NC	1
257		15	max	0	1	.001	2	0	3	3.314e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.199e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.644e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.143e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.973e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-1.087e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.303e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.032e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.399e-5	2	NC	1	NC	1
266			min	0	1	0	1	0	1	-9.759e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	4.539e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.112e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	2	3.04e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.116e-4	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	1.54e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.92e-4	1	NC	1	NC	1
273		4	max	0	3	0	2	0	10	4.013e-7	3	NC	1	NC	1
274			min	0	2	-.003	3	0	1	-2.725e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	-9.944e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	-.001	1	-3.529e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	15	-1.593e-5	15	NC	1	NC	1
278			min	0	2	-.004	3	-.002	1	-4.334e-4	1	NC	1	NC	1
279		7	max	0	3	.001	2	0	15	-1.895e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280		min	0	2	-.005	3	-.003	1	-5.138e-4	1	NC	1	NC	1
281	8	max	0	3	.001	2	0	15	-2.197e-5	15	NC	1	NC	1
282		min	0	2	-.006	3	-.004	1	-5.943e-4	1	NC	1	NC	1
283	9	max	0	3	.002	2	0	15	-2.499e-5	15	NC	1	NC	2
284		min	0	2	-.006	3	-.005	1	-6.747e-4	1	NC	1	8408.958	1
285	10	max	0	3	.002	2	0	15	-2.801e-5	15	NC	1	NC	2
286		min	0	2	-.007	3	-.007	1	-7.552e-4	1	NC	1	6761.382	1
287	11	max	0	3	.003	2	0	15	-3.103e-5	15	NC	1	NC	2
288		min	0	2	-.007	3	-.008	1	-8.357e-4	1	NC	1	5602.422	1
289	12	max	0	3	.003	2	0	15	-3.405e-5	15	NC	1	NC	2
290		min	0	2	-.007	3	-.01	1	-9.161e-4	1	NC	1	4756.207	1
291	13	max	0	3	.004	2	0	15	-3.707e-5	15	NC	1	NC	2
292		min	0	2	-.008	3	-.011	1	-9.966e-4	1	NC	1	4120.054	1
293	14	max	.001	3	.005	2	0	15	-4.009e-5	15	NC	1	NC	2
294		min	0	2	-.008	3	-.013	1	-1.077e-3	1	9320.872	2	3630.691	1
295	15	max	.001	3	.006	2	0	15	-4.311e-5	15	NC	1	NC	3
296		min	0	2	-.008	3	-.014	1	-1.157e-3	1	7921.913	2	3247.395	1
297	16	max	.001	3	.007	2	0	15	-4.613e-5	15	NC	3	NC	3
298		min	0	2	-.008	3	-.016	1	-1.238e-3	1	6819.077	2	2943.011	1
299	17	max	.001	3	.008	2	0	15	-4.915e-5	15	NC	3	NC	3
300		min	-.001	2	-.008	3	-.017	1	-1.318e-3	1	5943.161	2	2698.903	1
301	18	max	.001	3	.009	2	0	15	-5.217e-5	15	NC	3	NC	3
302		min	-.001	2	-.008	3	-.018	1	-1.399e-3	1	5242.861	2	2501.989	1
303	19	max	.001	3	.01	2	0	15	-5.519e-5	15	NC	3	NC	3
304		min	-.001	2	-.008	3	-.02	1	-1.479e-3	1	4680.09	2	2342.931	1
305	M12	1	max	.003	1	.01	.016	1	1.339e-3	1	NC	1	NC	3
306		min	0	3	-.008	3	0	15	5.066e-5	15	NC	1	1194.981	1
307	2	max	.002	1	.01	2	.015	1	1.339e-3	1	NC	1	NC	3
308		min	0	3	-.008	3	0	15	5.066e-5	15	NC	1	1303.113	1
309	3	max	.002	1	.009	2	.013	1	1.339e-3	1	NC	1	NC	3
310		min	0	3	-.007	3	0	15	5.066e-5	15	NC	1	1431.831	1
311	4	max	.002	1	.008	2	.012	1	1.339e-3	1	NC	1	NC	3
312		min	0	3	-.007	3	0	15	5.066e-5	15	NC	1	1586.561	1
313	5	max	.002	1	.008	2	.011	1	1.339e-3	1	NC	1	NC	3
314		min	0	3	-.006	3	0	15	5.066e-5	15	NC	1	1774.693	1
315	6	max	.002	1	.007	2	.01	1	1.339e-3	1	NC	1	NC	3
316		min	0	3	-.006	3	0	15	5.066e-5	15	NC	1	2006.508	1
317	7	max	.002	1	.007	2	.008	1	1.339e-3	1	NC	1	NC	3
318		min	0	3	-.005	3	0	15	5.066e-5	15	NC	1	2296.65	1
319	8	max	.002	1	.006	2	.007	1	1.339e-3	1	NC	1	NC	3
320		min	0	3	-.005	3	0	15	5.066e-5	15	NC	1	2666.559	1
321	9	max	.001	1	.006	2	.006	1	1.339e-3	1	NC	1	NC	3
322		min	0	3	-.004	3	0	15	5.066e-5	15	NC	1	3148.673	1
323	10	max	.001	1	.005	2	.005	1	1.339e-3	1	NC	1	NC	2
324		min	0	3	-.004	3	0	15	5.066e-5	15	NC	1	3793.986	1
325	11	max	.001	1	.004	2	.004	1	1.339e-3	1	NC	1	NC	2
326		min	0	3	-.004	3	0	15	5.066e-5	15	NC	1	4686.491	1
327	12	max	0	1	.004	2	.003	1	1.339e-3	1	NC	1	NC	2
328		min	0	3	-.003	3	0	15	5.066e-5	15	NC	1	5972.717	1
329	13	max	0	1	.003	2	.002	1	1.339e-3	1	NC	1	NC	2
330		min	0	3	-.003	3	0	15	5.066e-5	15	NC	1	7927.544	1
331	14	max	0	1	.003	2	.002	1	1.339e-3	1	NC	1	NC	1
332		min	0	3	-.002	3	0	15	5.066e-5	15	NC	1	NC	1
333	15	max	0	1	.002	2	.001	1	1.339e-3	1	NC	1	NC	1
334		min	0	3	-.002	3	0	15	5.066e-5	15	NC	1	NC	1
335	16	max	0	1	.002	2	0	1	1.339e-3	1	NC	1	NC	1
336		min	0	3	-.001	3	0	15	5.066e-5	15	NC	1	NC	1



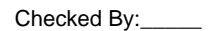
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	1.339e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	5.066e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.339e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	5.066e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.339e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	5.066e-5	15	NC	1	NC	1
343	M1	1	max	.008	3	.024	3	.001	3	1.813e-2	1	NC	1	NC	1
344			min	-.008	2	-.028	1	-.006	1	-1.699e-2	3	NC	1	NC	1
345		2	max	.007	3	.014	3	0	3	8.533e-3	1	NC	4	NC	2
346			min	-.008	2	-.016	1	-.014	1	-8.418e-3	3	3665.668	1	6056.259	1
347		3	max	.007	3	.004	3	0	3	-5.971e-6	12	NC	4	NC	3
348			min	-.008	2	-.004	1	-.019	1	-8.803e-4	1	1895.719	1	3671.352	1
349		4	max	.007	3	.006	1	0	3	-4.597e-7	3	NC	5	NC	3
350			min	-.008	2	-.004	3	-.021	1	-7.43e-4	1	1341.004	1	3036.737	1
351		5	max	.007	3	.015	1	0	12	5.544e-6	3	NC	5	NC	3
352			min	-.008	2	-.011	3	-.022	1	-6.058e-4	1	1074.417	1	2914.239	1
353		6	max	.007	3	.022	1	0	12	1.155e-5	3	NC	5	NC	3
354			min	-.008	2	-.016	3	-.021	1	-4.685e-4	1	923.777	1	3115.435	1
355		7	max	.007	3	.027	1	0	12	1.755e-5	3	NC	5	NC	3
356			min	-.008	2	-.02	3	-.018	1	-3.312e-4	1	832.617	1	3703.848	1
357		8	max	.007	3	.031	1	0	12	2.355e-5	3	NC	5	NC	2
358			min	-.008	2	-.023	3	-.015	1	-1.939e-4	1	777.458	1	5069.025	1
359		9	max	.007	3	.033	1	0	3	2.956e-5	3	NC	5	NC	2
360			min	-.008	2	-.024	3	-.01	1	-5.667e-5	1	747.34	1	9094.486	1
361		10	max	.007	3	.034	1	0	3	8.06e-5	1	NC	5	NC	1
362			min	-.008	2	-.025	3	-.006	1	3.316e-6	15	737.211	1	NC	1
363		11	max	.007	3	.033	1	0	3	2.179e-4	1	NC	5	NC	1
364			min	-.008	2	-.024	3	-.001	1	8.349e-6	15	745.54	1	NC	1
365		12	max	.007	3	.031	1	.003	1	3.551e-4	1	NC	5	NC	2
366			min	-.008	2	-.022	3	0	15	1.338e-5	15	773.684	1	5934.153	1
367		13	max	.007	3	.027	1	.006	1	4.924e-4	1	NC	5	NC	2
368			min	-.008	2	-.019	3	0	15	1.841e-5	15	826.46	1	4112.94	1
369		14	max	.007	3	.021	1	.009	1	6.297e-4	1	NC	5	NC	3
370			min	-.008	2	-.015	3	0	15	2.345e-5	15	914.43	1	3369.309	1
371		15	max	.007	3	.014	1	.01	1	7.67e-4	1	NC	5	NC	3
372			min	-.008	2	-.01	3	0	15	2.848e-5	15	1060.245	1	3101.852	1
373		16	max	.007	3	.005	1	.01	1	8.643e-4	1	NC	5	NC	3
374			min	-.008	2	-.004	3	0	15	3.208e-5	15	1317.944	1	3196.066	1
375		17	max	.007	3	.003	3	.008	1	2.862e-5	3	NC	4	NC	3
376			min	-.008	2	-.006	2	0	15	-3.184e-6	2	1850.158	1	3833.997	1
377		18	max	.007	3	.011	3	.003	1	1.018e-2	1	NC	4	NC	2
378			min	-.008	2	-.018	2	0	15	-3.722e-3	3	3566.193	1	6290.086	1
379		19	max	.007	3	.019	3	0	3	2.055e-2	1	NC	1	NC	1
380			min	-.008	2	-.031	2	-.004	1	-7.543e-3	3	NC	1	NC	1
381	M5	1	max	.022	3	.073	3	.001	3	8.56e-7	1	NC	1	NC	1
382			min	-.027	2	-.087	1	-.007	1	5.56e-8	15	NC	1	NC	1
383		2	max	.022	3	.042	3	.002	3	5.771e-5	3	NC	5	NC	1
384			min	-.028	2	-.049	1	-.006	1	-8.051e-5	1	1215.194	1	NC	1
385		3	max	.022	3	.013	3	.003	3	1.136e-4	3	NC	5	NC	1
386			min	-.028	2	-.013	1	-.005	1	-1.602e-4	1	625.358	1	NC	1
387		4	max	.022	3	.017	1	.003	3	1.111e-4	3	NC	5	NC	1
388			min	-.028	2	-.011	3	-.004	1	-1.501e-4	1	440.959	1	NC	1
389		5	max	.022	3	.044	1	.003	3	1.086e-4	3	NC	15	NC	1
390			min	-.028	2	-.03	3	-.004	1	-1.401e-4	1	352.25	1	NC	1
391		6	max	.022	3	.065	1	.004	3	1.061e-4	3	NC	15	NC	1
392			min	-.028	2	-.046	3	-.003	1	-1.3e-4	1	302.007	1	NC	1
393		7	max	.021	3	.082	1	.004	3	1.036e-4	3	NC	15	NC	1







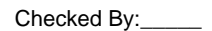
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.007	3	.003	3	0	15	-4.114e-6	12	NC	4	NC	3
452			min	-.008	2	-.006	2	-.016	1	-5.761e-4	1	1850.074	1	3544.526	1
453		18	max	.007	3	.011	3	0	15	3.735e-3	3	NC	4	NC	2
454			min	-.008	2	-.018	2	-.011	1	-1.046e-2	1	3566.032	1	5907.1	1
455		19	max	.007	3	.019	3	0	3	7.543e-3	3	NC	1	NC	1
456			min	-.008	2	-.031	2	-.002	1	-2.055e-2	1	NC	1	NC	1
457	M13	1	max	.008	1	.024	3	.008	3	4.002e-3	3	NC	1	NC	1
458			min	0	3	-.028	1	-.008	2	-4.822e-3	1	NC	1	NC	1
459		2	max	.008	1	.21	3	.063	1	4.846e-3	3	NC	5	NC	3
460			min	-.001	3	-.227	1	.002	10	-5.881e-3	1	1028.607	1	2985.805	1
461		3	max	.008	1	.362	3	.16	1	5.689e-3	3	NC	5	NC	3
462			min	-.001	3	-.389	1	.006	15	-6.941e-3	1	565.814	1	1239.138	1
463		4	max	.008	1	.457	3	.242	1	6.533e-3	3	NC	5	NC	3
464			min	-.001	3	-.491	1	.009	15	-8.e-3	1	441.264	1	827.132	1
465		5	max	.008	1	.484	3	.282	1	7.377e-3	3	NC	15	NC	3
466			min	-.001	3	-.521	1	.011	15	-9.06e-3	1	414.527	1	710.223	1
467		6	max	.008	1	.445	3	.27	1	8.22e-3	3	NC	5	NC	3
468			min	-.001	3	-.48	1	.01	15	-1.012e-2	1	451.976	1	743.155	1
469		7	max	.007	1	.352	3	.207	1	9.064e-3	3	NC	5	NC	3
470			min	-.001	3	-.383	1	.008	15	-1.118e-2	1	575.992	1	963.572	1
471		8	max	.007	1	.232	3	.112	1	9.908e-3	3	NC	5	NC	3
472			min	-.001	3	-.256	1	0	10	-1.224e-2	1	896.197	1	1739.132	1
473		9	max	.007	1	.123	3	.021	3	1.075e-2	3	NC	5	NC	2
474			min	-.001	3	-.14	1	-.011	10	-1.33e-2	1	1827.891	1	8193.822	1
475		10	max	.007	1	.073	3	.022	3	1.159e-2	3	NC	4	NC	1
476			min	-.001	3	-.087	1	-.027	2	-1.436e-2	1	3467.862	1	NC	1
477		11	max	.007	1	.123	3	.026	1	1.075e-2	3	NC	5	NC	2
478			min	-.001	3	-.14	1	-.011	10	-1.33e-2	1	1827.893	1	6487.97	1
479		12	max	.007	1	.232	3	.123	1	9.908e-3	3	NC	5	NC	5
480			min	-.001	3	-.256	1	0	10	-1.224e-2	1	896.198	1	1593.479	1
481		13	max	.007	1	.352	3	.22	1	9.065e-3	3	NC	5	NC	5
482			min	-.001	3	-.383	1	.008	15	-1.118e-2	1	575.992	1	908.466	1
483		14	max	.007	1	.445	3	.283	1	8.222e-3	3	NC	5	NC	5
484			min	-.001	3	-.48	1	.011	15	-1.012e-2	1	451.977	1	709.007	1
485		15	max	.006	1	.484	3	.294	1	7.379e-3	3	NC	15	NC	5
486			min	-.001	3	-.52	1	.011	15	-9.058e-3	1	414.527	1	681.347	1
487		16	max	.006	1	.457	3	.252	1	6.536e-3	3	NC	5	NC	5
488			min	-.001	3	-.491	1	.009	15	-7.998e-3	1	441.265	1	794.864	1
489		17	max	.006	1	.362	3	.167	1	5.692e-3	3	NC	5	NC	3
490			min	-.001	3	-.389	1	.006	15	-6.938e-3	1	565.815	1	1188.226	1
491		18	max	.006	1	.21	3	.067	1	4.849e-3	3	NC	5	NC	3
492			min	-.001	3	-.227	1	.002	10	-5.878e-3	1	1028.608	1	2835.899	1
493		19	max	.006	1	.024	3	.008	3	4.006e-3	3	NC	1	NC	1
494			min	-.001	3	-.028	1	-.008	2	-4.819e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.019	3	.007	3	5.052e-3	1	NC	1	NC	1
496			min	0	3	-.031	2	-.008	2	-3.081e-3	3	NC	1	NC	1
497		2	max	.002	1	.102	3	.068	1	6.207e-3	1	NC	5	NC	3
498			min	0	3	-.256	1	.002	10	-3.7e-3	3	907.344	1	2770.453	1
499		3	max	.003	1	.17	3	.168	1	7.363e-3	1	NC	5	NC	3
500			min	0	3	-.439	1	.006	15	-4.319e-3	3	499.128	1	1174.325	1
501		4	max	.003	1	.213	3	.253	1	8.518e-3	1	NC	5	NC	5
502			min	0	3	-.555	1	.01	15	-4.938e-3	3	389.282	1	790.557	1
503		5	max	.003	1	.227	3	.294	1	9.674e-3	1	NC	15	NC	5
504			min	0	3	-.589	1	.011	15	-5.557e-3	3	365.734	1	680.901	1
505		6	max	.003	1	.211	3	.281	1	1.083e-2	1	NC	5	NC	5
506			min	0	3	-.542	1	.011	15	-6.176e-3	3	398.85	1	711.911	1
507		7	max	.003	1	.173	3	.217	1	1.199e-2	1	NC	5	NC	5



***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.001	1	-.014	15	.011	1	8.373e-3	3	7697.042	15	NC	4
566			min	0	10	-.061	4	-.008	3	-1.075e-2	1	1809.305	4	3119.555	1
567		18	max	.001	1	-.007	15	.007	3	8.875e-3	3	NC	15	NC	4
568			min	0	10	-.032	4	-.011	2	-1.142e-2	1	3555.563	4	5556.197	1
569		19	max	.001	1	.003	3	.026	3	9.377e-3	3	NC	1	NC	1
570			min	0	10	-.004	1	-.03	2	-1.209e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	10	.009	3	3.153e-3	3	NC	1	NC	1
572			min	-.001	1	-.002	1	-.009	2	-3.517e-3	1	NC	1	NC	1
573		2	max	0	10	-.007	15	.007	1	3.016e-3	3	NC	15	NC	2
574			min	-.001	1	-.031	4	-.001	10	-3.348e-3	1	3555.563	4	8312.084	1
575		3	max	0	10	-.014	15	.017	1	2.88e-3	3	7697.042	15	NC	4
576			min	-.001	1	-.061	4	-.004	3	-3.178e-3	1	1809.305	4	4699.855	1
577		4	max	0	10	-.021	15	.025	1	2.743e-3	3	5280.621	15	NC	4
578			min	-.001	1	-.088	4	-.008	3	-3.009e-3	1	1241.289	4	3571.893	1
579		5	max	0	10	-.026	15	.03	1	2.606e-3	3	4120.522	15	NC	4
580			min	-.001	1	-.112	4	-.01	3	-2.839e-3	1	968.59	4	3082.174	1
581		6	max	0	10	-.031	15	.033	1	2.47e-3	3	3467.855	15	NC	4
582			min	0	1	-.133	4	-.012	3	-2.67e-3	1	815.171	4	2867.078	1
583		7	max	0	10	-.035	15	.034	1	2.333e-3	3	3075.361	15	NC	4
584			min	0	1	-.15	4	-.013	3	-2.5e-3	1	722.91	4	2812.525	1
585		8	max	0	10	-.038	15	.034	1	2.196e-3	3	2839.807	15	NC	4
586			min	0	1	-.163	4	-.013	3	-2.331e-3	1	667.539	4	2879.284	1
587		9	max	0	10	-.04	15	.032	1	2.06e-3	3	2713.017	15	NC	4
588			min	0	1	-.17	4	-.012	3	-2.161e-3	1	637.735	4	3061.645	1
589		10	max	0	10	-.041	15	.029	1	1.923e-3	3	2672.907	15	NC	4
590			min	0	1	-.173	4	-.011	3	-1.992e-3	1	628.307	4	3377.71	1
591		11	max	0	10	-.04	15	.025	1	1.786e-3	3	2713.017	15	NC	4
592			min	0	1	-.17	4	-.01	3	-1.822e-3	1	637.735	4	3873.188	1
593		12	max	0	10	-.038	15	.021	1	1.65e-3	3	2839.807	15	NC	4
594			min	0	1	-.163	4	-.008	3	-1.653e-3	1	667.539	4	4638.902	1
595		13	max	0	10	-.035	15	.016	1	1.513e-3	3	3075.361	15	NC	3
596			min	0	1	-.15	4	-.006	3	-1.497e-3	2	722.91	4	5855.696	1
597		14	max	0	10	-.031	15	.012	1	1.376e-3	3	3467.855	15	NC	2
598			min	0	1	-.133	4	-.004	3	-1.344e-3	2	815.171	4	7913.362	1
599		15	max	0	10	-.026	15	.008	1	1.24e-3	3	4120.522	15	NC	1
600			min	0	1	-.112	4	-.002	3	-1.191e-3	2	968.59	4	NC	1
601		16	max	0	10	-.021	15	.004	1	1.103e-3	3	5280.621	15	NC	1
602			min	0	1	-.087	4	0	3	-1.037e-3	2	1241.289	4	NC	1
603		17	max	0	10	-.014	15	.002	1	9.664e-4	3	7697.042	15	NC	1
604			min	0	1	-.06	4	0	10	-8.842e-4	2	1809.305	4	NC	1
605		18	max	0	10	-.007	15	0	4	8.297e-4	3	NC	15	NC	1
606			min	0	1	-.031	4	0	2	-7.311e-4	2	3555.563	4	NC	1
607		19	max	0	1	0	1	0	1	6.93e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-5.779e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

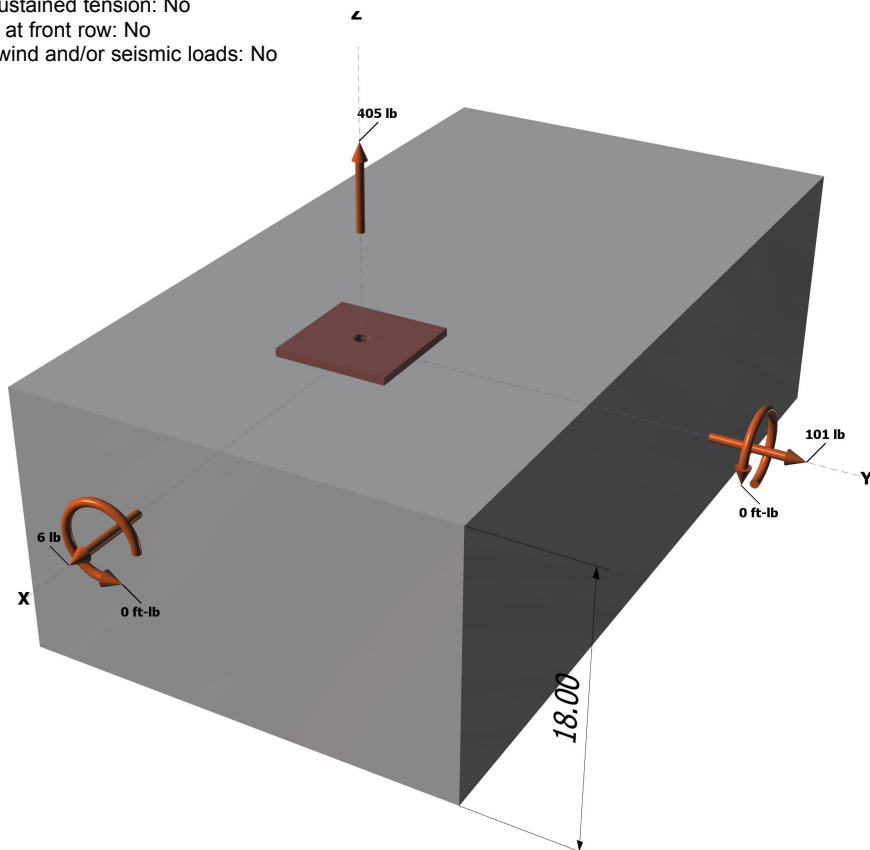
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

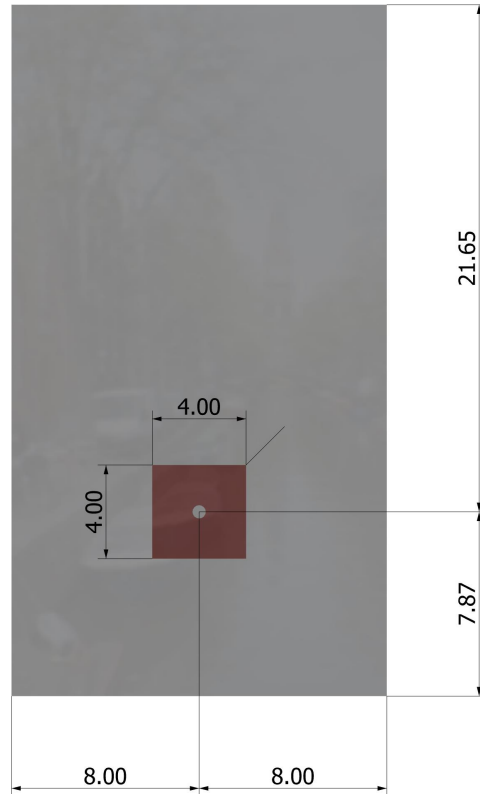
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

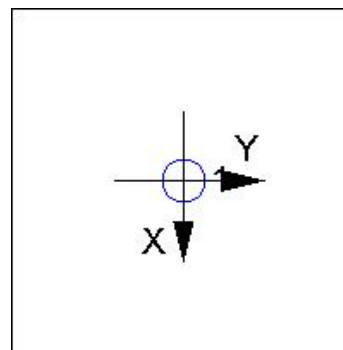
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

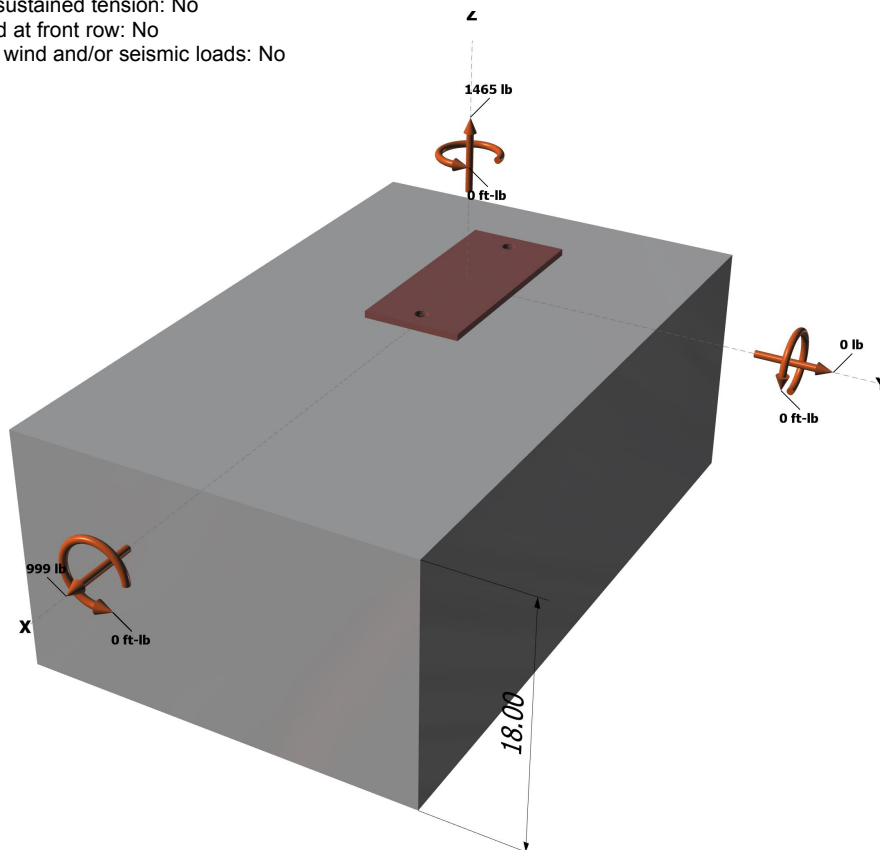
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



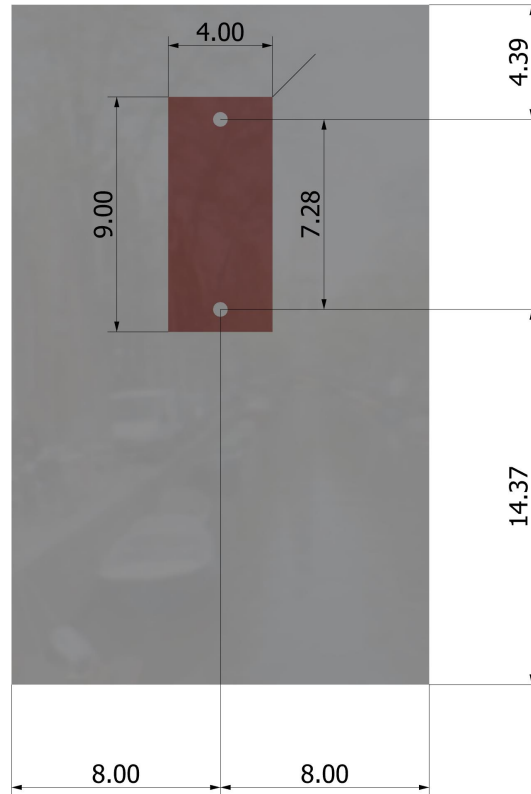
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.