



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

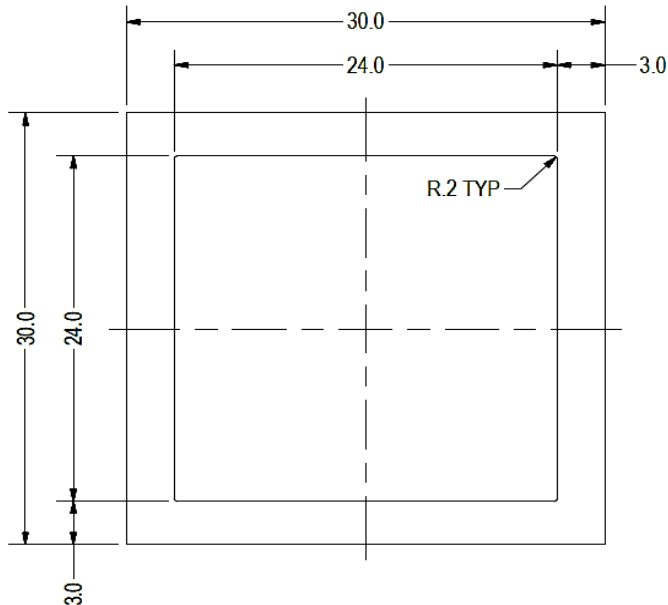
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.047 k-ft
P_n =	0.222 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	13%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.771 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.806 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.235 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	15%



A cross brace kit is required every 13 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

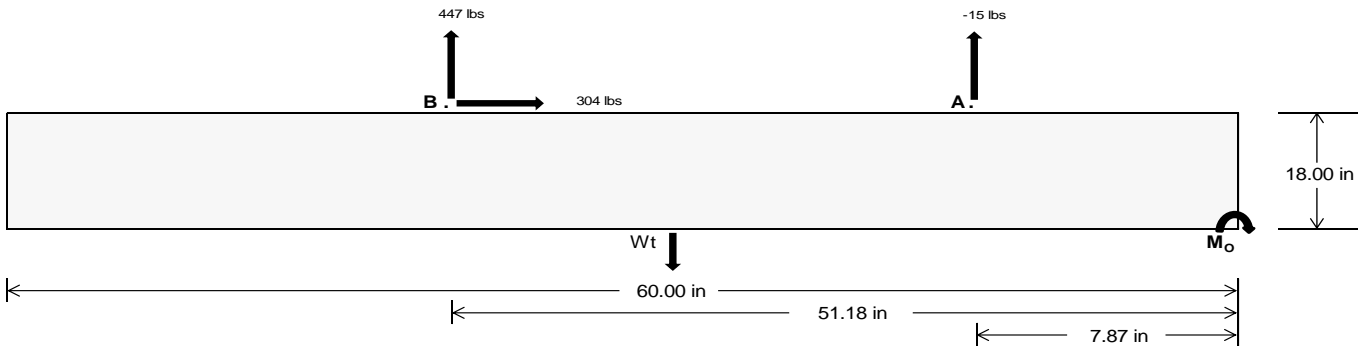
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	17.09	1861.01	k
Compressive Load =	1224.22	1346.85	k
Lateral Load =	38.29	1265.64	k
Moment (Weak Axis) =	0.06	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 28215.9$ in-lbs
Resisting Force Required = 940.53 lbs
S.F. = 1.67
Weight Required = 1567.55 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 304.09 lbs
Friction = 0.4
Weight Required = 760.22 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 304.09 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	480 lbs	480 lbs	480 lbs	480 lbs	362 lbs	362 lbs	362 lbs	362 lbs	583 lbs	583 lbs	583 lbs	583 lbs	30 lbs	30 lbs	30 lbs	30 lbs
F_B	323 lbs	323 lbs	323 lbs	323 lbs	584 lbs	584 lbs	584 lbs	584 lbs	647 lbs	647 lbs	647 lbs	647 lbs	-893 lbs	-893 lbs	-893 lbs	-893 lbs
F_V	62 lbs	62 lbs	62 lbs	62 lbs	556 lbs	556 lbs	556 lbs	556 lbs	458 lbs	458 lbs	458 lbs	458 lbs	-608 lbs	-608 lbs	-608 lbs	-608 lbs
P_{total}	2797 lbs	2888 lbs	2979 lbs	3069 lbs	2939 lbs	3030 lbs	3121 lbs	3211 lbs	3224 lbs	3315 lbs	3405 lbs	3496 lbs	333 lbs	388 lbs	442 lbs	496 lbs
M	408 lbs-ft	408 lbs-ft	408 lbs-ft	408 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.19 ft	0.19 ft	0.19 ft	0.18 ft	2.16 ft	1.86 ft	1.63 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	251.7 psf	250.2 psf	248.9 psf	247.6 psf	259.0 psf	257.2 psf	255.5 psf	254.0 psf	270.3 psf	268.0 psf	265.9 psf	264.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	358.6 psf	352.5 psf	346.8 psf	341.7 psf	382.4 psf	375.2 psf	368.6 psf	362.6 psf	433.1 psf	423.7 psf	415.1 psf	407.2 psf	359.0 psf	210.4 psf	169.4 psf	151.5 psf

Maximum Bearing Pressure = 433 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

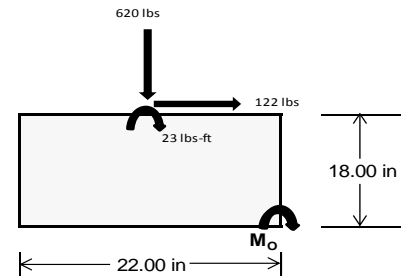
Overturning Check

$M_o = 361.6 \text{ ft-lbs}$
 Resisting Force Required = 394.47 lbs
 S.F. = 1.67
 Weight Required = 657.45 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	145 lbs	132 lbs	82 lbs	293 lbs	620 lbs	245 lbs	88 lbs	-11 lbs	28 lbs
F_h	20 lbs	162 lbs	21 lbs	14 lbs	122 lbs	16 lbs	21 lbs	162 lbs	21 lbs
P_{total}	2614 lbs	2600 lbs	2551 lbs	2643 lbs	2969 lbs	2594 lbs	810 lbs	711 lbs	750 lbs
M	59 lbs-ft	275 lbs-ft	63 lbs-ft	39 lbs-ft	207 lbs-ft	50 lbs-ft	60 lbs-ft	274 lbs-ft	63 lbs-ft
e	0.02 ft	0.11 ft	0.02 ft	0.01 ft	0.07 ft	0.02 ft	0.07 ft	0.39 ft	0.08 ft
$L/6$	0.31 ft	1.62 ft	1.78 ft	1.80 ft	1.69 ft	1.80 ft	1.68 ft	1.06 ft	1.67 ft
f_{min}	264.2 sqft	185.6 sqft	255.8 sqft	274.6 sqft	250.2 sqft	265.3 sqft	66.8 sqft	-20.4 sqft	59.5 sqft
f_{max}	306.1 psf	381.8 psf	300.6 psf	302.1 psf	397.7 psf	300.7 psf	109.9 psf	175.5 psf	104.1 psf



Maximum Bearing Pressure = 398 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.454 k
Allowable Uplift =	1.214 k
Utilization =	<u>37%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.088 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.942 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Diagonal Strut

Maximum Axial Load =	0.771 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.164 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.235 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.099 in
	<u>0.099 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$210.919$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$219.027$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.218 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1371 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.446 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

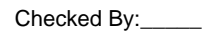
APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	24.642	15	0	1	-28.954	4	0	1	-.01	4	0	1
87		6	max	375.772	1	0	1	-.213	10	0	1	0	12	0	1
88			min	24.661	15	0	1	-29.01	4	0	1	-.013	4	0	1
89		7	max	375.837	1	0	1	-.213	10	0	1	0	12	0	1
90			min	24.681	15	0	1	-29.066	4	0	1	-.015	4	0	1
91		8	max	375.902	1	0	1	-.213	10	0	1	0	12	0	1
92			min	24.7	15	0	1	-29.122	4	0	1	-.018	4	0	1
93		9	max	375.966	1	0	1	-.213	10	0	1	0	12	0	1
94			min	24.72	15	0	1	-29.178	4	0	1	-.021	4	0	1
95		10	max	376.031	1	0	1	-.213	10	0	1	0	12	0	1
96			min	24.739	15	0	1	-29.234	4	0	1	-.023	4	0	1
97		11	max	376.096	1	0	1	-.213	10	0	1	0	12	0	1
98			min	24.759	15	0	1	-29.291	4	0	1	-.026	4	0	1
99		12	max	376.16	1	0	1	-.213	10	0	1	0	12	0	1
100			min	24.778	15	0	1	-29.347	4	0	1	-.029	4	0	1
101		13	max	376.225	1	0	1	-.213	10	0	1	0	12	0	1
102			min	24.798	15	0	1	-29.403	4	0	1	-.031	4	0	1
103		14	max	376.29	1	0	1	-.213	10	0	1	0	12	0	1
104			min	24.817	15	0	1	-29.459	4	0	1	-.034	4	0	1
105		15	max	376.354	1	0	1	-.213	10	0	1	0	12	0	1
106			min	24.837	15	0	1	-29.515	4	0	1	-.036	4	0	1
107		16	max	376.419	1	0	1	-.213	10	0	1	0	12	0	1
108			min	24.856	15	0	1	-29.571	4	0	1	-.039	4	0	1
109		17	max	376.484	1	0	1	-.213	10	0	1	0	12	0	1
110			min	24.876	15	0	1	-29.627	4	0	1	-.042	4	0	1
111		18	max	376.549	1	0	1	-.213	10	0	1	0	12	0	1
112			min	24.895	15	0	1	-29.683	4	0	1	-.044	4	0	1
113		19	max	376.613	1	0	1	-.213	10	0	1	0	12	0	1
114			min	24.915	15	0	1	-29.739	4	0	1	-.047	4	0	1
115	M6	1	max	803.773	1	.664	6	1.274	4	0	3	0	3	0	1
116			min	-1163.955	3	.148	15	-.159	3	0	5	0	11	0	1
117		2	max	803.908	1	.607	6	1.151	4	0	3	0	4	0	15
118			min	-1163.853	3	.134	15	-.159	3	0	5	0	11	0	6
119		3	max	804.042	1	.549	6	1.027	4	0	3	0	4	0	15
120			min	-1163.752	3	.121	15	-.159	3	0	5	0	11	0	6
121		4	max	804.177	1	.492	6	.904	4	0	3	0	4	0	15
122			min	-1163.651	3	.107	15	-.159	3	0	5	0	10	0	6
123		5	max	804.312	1	.446	2	.781	4	0	3	0	4	0	15
124			min	-1163.55	3	.094	15	-.159	3	0	5	0	10	0	6
125		6	max	804.447	1	.401	2	.658	4	0	3	.001	4	0	15
126			min	-1163.449	3	.075	12	-.159	3	0	5	0	10	0	6
127		7	max	804.582	1	.357	2	.535	4	0	3	.001	4	0	15
128			min	-1163.348	3	.052	12	-.159	3	0	5	0	10	0	6
129		8	max	804.717	1	.312	2	.412	4	0	3	.001	4	0	15
130			min	-1163.247	3	.03	12	-.159	3	0	5	0	3	0	2
131		9	max	804.852	1	.267	2	.288	4	0	3	.001	4	0	15
132			min	-1163.145	3	.004	3	-.159	3	0	5	0	3	0	2
133		10	max	804.986	1	.222	2	.165	4	0	3	.001	4	0	12
134			min	-1163.044	3	-.03	3	-.159	3	0	5	0	3	0	2
135		11	max	805.121	1	.177	2	.137	1	0	3	.001	4	0	12
136			min	-1162.943	3	-.063	3	-.159	3	0	5	0	3	0	2
137		12	max	805.256	1	.133	2	.137	1	0	3	.001	4	0	12
138			min	-1162.842	3	-.097	3	-.159	3	0	5	0	3	0	2
139		13	max	805.391	1	.088	2	.137	1	0	3	.001	4	0	12
140			min	-1162.741	3	-.131	3	-.244	5	0	5	0	3	0	2
141		14	max	805.526	1	.043	2	.137	1	0	3	.001	4	0	12
142			min	-1162.64	3	-.164	3	-.368	5	0	5	0	3	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	805.661	1	-0.002	2	.137	1	0	3	.001	4	0	12
144		min	-1162.539	3	-.198	3	-.491	5	0	5	0	3	0	2
145	16	max	805.796	1	-.046	2	.137	1	0	3	.001	4	0	12
146		min	-1162.437	3	-.231	3	-.614	5	0	5	0	3	0	2
147	17	max	805.931	1	-.069	15	.137	1	0	3	.001	4	0	3
148		min	-1162.336	3	-.265	3	-.737	5	0	5	0	3	0	2
149	18	max	806.065	1	-.082	15	.137	1	0	3	0	4	0	3
150		min	-1162.235	3	-.314	4	-.86	5	0	5	0	3	0	2
151	19	max	806.2	1	-.096	15	.137	1	0	3	0	4	0	3
152		min	-1162.134	3	-.371	4	-.983	5	0	5	0	3	0	2
153	M7	1	max	770.5	2	1.756	.03	3	0	14	0	4	0	2
154		min	-668.996	3	.42	15	-1.291	5	0	3	0	3	0	3
155	2	max	770.43	2	1.58	4	.03	3	0	14	0	4	0	2
156		min	-669.048	3	.378	15	-1.157	5	0	3	0	3	0	3
157	3	max	770.36	2	1.403	4	.03	3	0	14	0	2	0	2
158		min	-669.101	3	.337	15	-1.023	5	0	3	0	3	0	3
159	4	max	770.29	2	1.227	4	.03	3	0	14	0	2	0	2
160		min	-669.153	3	.295	15	-.89	5	0	3	0	5	0	3
161	5	max	770.22	2	1.051	4	.03	3	0	14	0	2	0	15
162		min	-669.206	3	.254	15	-.756	5	0	3	0	5	0	3
163	6	max	770.15	2	.874	4	.03	3	0	14	0	2	0	15
164		min	-669.258	3	.212	15	-.622	5	0	3	0	5	0	3
165	7	max	770.08	2	.698	4	.03	3	0	14	0	2	0	15
166		min	-669.311	3	.171	15	-.489	5	0	3	0	5	0	6
167	8	max	770.01	2	.521	4	.03	3	0	14	0	2	0	15
168		min	-669.363	3	.121	12	-.355	5	0	3	0	5	-.001	6
169	9	max	769.94	2	.348	2	.03	3	0	14	0	2	0	15
170		min	-669.416	3	.052	12	-.221	5	0	3	0	5	-.001	6
171	10	max	769.87	2	.21	2	.03	3	0	14	0	2	0	15
172		min	-669.468	3	-.032	3	-.088	5	0	3	0	5	-.001	6
173	11	max	769.8	2	.073	2	.047	4	0	14	0	2	0	15
174		min	-669.521	3	-.136	3	-.008	2	0	3	0	5	-.001	6
175	12	max	769.73	2	-.036	15	.181	4	0	14	0	2	0	15
176		min	-669.573	3	-.239	3	-.008	2	0	3	0	5	-.001	6
177	13	max	769.66	2	-.078	15	.315	4	0	14	0	2	0	15
178		min	-669.626	3	-.361	6	-.008	2	0	3	0	5	-.001	6
179	14	max	769.59	2	-.119	15	.448	4	0	14	0	11	0	15
180		min	-669.678	3	-.537	6	-.008	2	0	3	0	5	-.001	6
181	15	max	769.52	2	-.161	15	.582	4	0	14	0	11	0	15
182		min	-669.731	3	-.714	6	-.008	2	0	3	0	5	0	6
183	16	max	769.45	2	-.202	15	.716	4	0	14	0	11	0	15
184		min	-669.783	3	-.89	6	-.008	2	0	3	0	5	0	6
185	17	max	769.38	2	-.244	15	.849	4	0	14	0	11	0	15
186		min	-669.836	3	-1.067	6	-.008	2	0	3	0	5	0	6
187	18	max	769.31	2	-.285	15	.983	4	0	14	0	11	0	15
188		min	-669.888	3	-1.243	6	-.008	2	0	3	0	5	0	6
189	19	max	769.24	2	-.327	15	1.117	4	0	14	0	14	0	1
190		min	-669.941	3	-1.419	6	-.008	2	0	3	0	3	0	1
191	M8	1	max	940.542	1	0	.769	1	0	1	0	4	0	1
192		min	32.9	15	0	1	-28.749	4	0	1	0	1	0	1
193	2	max	940.607	1	0	1	.769	1	0	1	0	1	0	1
194		min	32.919	15	0	1	-28.805	4	0	1	-.003	4	0	1
195	3	max	940.672	1	0	1	.769	1	0	1	0	1	0	1
196		min	32.939	15	0	1	-28.861	4	0	1	-.005	4	0	1
197	4	max	940.736	1	0	1	.769	1	0	1	0	1	0	1
198		min	32.958	15	0	1	-28.917	4	0	1	-.008	4	0	1
199	5	max	940.801	1	0	1	.769	1	0	1	0	1	0	1







RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 35° 90mph 30psf 6.75ft 7-05.r Page 27





Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-14.846	10	-28.391	2	-26.726	5	0	5	-.069	1	-.141	3
429	6	max	115.235	3	5.931	9	64.962	1	0	1	.052	5	.122	2
430		min	-14.713	10	-28.62	2	-26.484	5	0	5	-.055	1	-.139	3
431	7	max	115.355	3	5.741	9	64.962	1	0	1	.047	5	.128	2
432		min	-14.58	10	-28.849	2	-26.242	5	0	5	-.04	1	-.137	3
433	8	max	115.476	3	5.55	9	64.962	1	0	1	.041	5	.134	2
434		min	-14.446	10	-29.078	2	-26	5	0	5	-.026	1	-.135	3
435	9	max	115.596	3	5.359	9	64.962	1	0	1	.035	5	.141	2
436		min	-14.313	10	-29.306	2	-25.758	5	0	5	-.012	1	-.133	3
437	10	max	115.716	3	5.169	9	64.962	1	0	1	.03	4	.147	2
438		min	-14.179	10	-29.535	2	-25.516	5	0	5	0	2	-.13	3
439	11	max	115.836	3	4.978	9	64.962	1	0	1	.028	4	.153	2
440		min	-14.046	10	-29.764	2	-25.274	5	0	5	.002	10	-.128	3
441	12	max	115.956	3	4.788	9	64.962	1	0	1	.03	1	.16	2
442		min	-13.912	10	-29.993	2	-25.032	5	0	5	.003	10	-.126	3
443	13	max	116.076	3	4.597	9	64.962	1	0	1	.044	1	.166	2
444		min	-13.779	10	-30.221	2	-24.79	5	0	5	.005	10	-.124	3
445	14	max	116.196	3	4.406	9	64.962	1	0	1	.058	1	.173	2
446		min	-13.645	10	-30.45	2	-24.548	5	0	5	.005	15	-.121	3
447	15	max	116.316	3	4.216	9	64.962	1	0	1	.072	1	.18	2
448		min	-13.512	10	-30.679	2	-24.306	5	0	5	.002	15	-.119	3
449	16	max	91.275	2	144.428	2	65.417	1	0	10	.087	1	.185	2
450		min	4.393	15	-201.568	3	-22.879	5	0	4	0	5	-.115	3
451	17	max	91.435	2	144.199	2	65.417	1	0	10	.101	1	.153	2
452		min	4.441	15	-201.739	3	-22.637	5	0	4	-.006	5	-.071	3
453	18	max	-.015	15	355.166	2	68.926	1	0	2	.116	1	.077	2
454		min	-139.016	1	-163.617	3	-48.622	5	0	3	-.016	5	-.036	3
455	19	max	.033	15	354.938	2	68.926	1	0	2	.131	1	0	2
456		min	-138.856	1	-163.788	3	-48.38	5	0	3	-.027	5	0	3
457	M13	1	max	192.193	4	245.006	1	-4.367	15	0	.133	1	0	1
458		min	7.347	10	-336.563	3	-138.944	1	0	3	.003	15	0	3
459	2	max	184.827	4	172.944	1	-2.896	15	0	2	.041	1	.215	3
460		min	7.347	10	-237.477	3	-106.366	1	0	3	0	15	-.157	1
461	3	max	177.461	4	100.883	1	-1.426	15	0	2	.004	3	.356	3
462		min	7.347	10	-138.39	3	-73.788	1	0	3	-.027	1	-.259	1
463	4	max	170.095	4	28.821	1	.045	15	0	2	0	3	.423	3
464		min	7.347	10	-39.304	3	-41.21	1	0	3	-.07	1	-.308	1
465	5	max	162.728	4	59.783	3	2.235	5	0	2	-.002	15	.415	3
466		min	7.347	10	-43.241	1	-8.632	1	0	3	-.089	1	-.303	1
467	6	max	155.362	4	158.869	3	23.946	1	0	2	0	15	.333	3
468		min	7.347	10	-115.302	1	-.091	3	0	3	-.083	1	-.243	1
469	7	max	147.996	4	257.956	3	56.524	1	0	2	.004	5	.177	3
470		min	7.347	10	-187.364	1	1.434	12	0	3	-.053	1	-.13	1
471	8	max	140.63	4	357.042	3	89.103	1	0	2	.01	4	.038	1
472		min	7.347	10	-259.426	1	2.861	12	0	3	0	3	-.054	3
473	9	max	133.264	4	456.128	3	121.681	1	0	2	.081	1	.259	1
474		min	7.347	10	-331.487	1	4.288	12	0	3	.002	12	-.359	3
475	10	max	125.897	4	555.215	3	154.259	1	0	2	.184	1	.535	1
476		min	7.347	10	-403.549	1	5.714	12	0	3	.006	12	-.738	3
477	11	max	93.159	4	331.487	1	-.552	15	0	3	.078	1	.259	1
478		min	4.115	12	-456.128	3	-121.071	1	0	2	-.014	5	-.359	3
479	12	max	85.793	4	259.426	1	1.131	5	0	3	.002	2	.038	1
480		min	4.115	12	-357.042	3	-88.493	1	0	2	-.014	4	-.054	3
481	13	max	78.426	4	187.364	1	3.406	5	0	3	-.005	12	.177	3
482		min	4.115	12	-257.955	3	-55.915	1	0	2	-.055	1	-.13	1
483	14	max	71.06	4	115.302	1	5.681	5	0	3	-.005	12	.333	3
484		min	4.115	12	-158.869	3	-23.337	1	0	2	-.084	1	-.243	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	68.026	1	43.241	1	10.27	4	0	3	-.002	15	.415	3
486		min	4.115	12	-59.783	3	.416	10	0	2	-.09	1	-.303	1
487	16	max	68.026	1	39.304	3	41.819	1	0	3	.004	5	.423	3
488		min	4.115	12	-28.821	1	3.069	12	0	2	-.07	1	-.308	1
489	17	max	68.026	1	138.39	3	74.397	1	0	3	.012	5	.356	3
490		min	4.115	12	-100.883	1	4.495	12	0	2	-.027	1	-.259	1
491	18	max	68.026	1	237.477	3	106.976	1	0	3	.041	1	.215	3
492		min	4.115	12	-172.944	1	5.922	12	0	2	.003	10	-.157	1
493	19	max	68.026	1	336.563	3	139.554	1	0	3	.134	1	0	1
494		min	4.115	12	-245.006	1	7.349	12	0	2	.009	12	0	3
495	M16	1	max	48.381	5	355.168	2	.033	15	0	.131	1	0	2
496		min	-68.682	1	-163.821	3	-138.871	1	0	2	-.027	5	0	3
497	2	max	41.015	5	250.724	2	1.948	5	0	3	.039	1	.105	3
498		min	-68.682	1	-115.806	3	-106.292	1	0	2	-.026	5	-.227	2
499	3	max	33.648	5	146.279	2	4.223	5	0	3	0	12	.174	3
500		min	-68.682	1	-67.792	3	-73.714	1	0	2	-.03	4	-.376	2
501	4	max	26.282	5	41.835	2	6.498	5	0	3	-.003	12	.207	3
502		min	-68.682	1	-19.777	3	-41.136	1	0	2	-.071	1	-.447	2
503	5	max	18.916	5	28.237	3	8.773	5	0	3	-.005	12	.203	3
504		min	-68.682	1	-62.61	2	-8.558	1	0	2	-.09	1	-.439	2
505	6	max	11.55	5	76.251	3	24.02	1	0	3	-.004	15	.164	3
506		min	-68.682	1	-167.054	2	.333	12	0	2	-.084	1	-.353	2
507	7	max	4.184	5	124.266	3	56.598	1	0	3	.003	5	.089	3
508		min	-68.682	1	-271.498	2	1.76	12	0	2	-.054	1	-.188	2
509	8	max	-1.046	12	172.28	3	89.176	1	0	3	.014	4	.055	2
510		min	-68.682	1	-375.943	2	3.186	12	0	2	-.004	3	-.022	3
511	9	max	-1.046	12	220.295	3	121.754	1	0	3	.08	1	.376	2
512		min	-68.682	1	-480.387	2	4.613	12	0	2	0	12	-.169	3
513	10	max	26.811	5	-11.759	15	154.332	1	0	14	.184	1	.775	2
514		min	-69.622	1	-584.831	2	-9.49	3	0	2	.007	12	-.353	3
515	11	max	19.445	5	480.387	2	-.586	15	0	2	.08	1	.376	2
516		min	-69.622	1	-220.295	3	-121.356	1	0	3	-.012	5	-.169	3
517	12	max	12.078	5	375.943	2	1.078	5	0	2	.002	2	.055	2
518		min	-69.622	1	-172.28	3	-88.778	1	0	3	-.012	4	-.022	3
519	13	max	4.712	5	271.498	2	3.353	5	0	2	-.002	12	.089	3
520		min	-69.622	1	-124.266	3	-56.2	1	0	3	-.054	1	-.188	2
521	14	max	-1.659	15	167.054	2	5.628	5	0	2	-.003	12	.164	3
522		min	-69.622	1	-76.251	3	-23.622	1	0	3	-.084	1	-.353	2
523	15	max	-4.398	12	62.609	2	10.192	4	0	2	-.001	15	.203	3
524		min	-69.622	1	-28.237	3	.428	10	0	3	-.089	1	-.439	2
525	16	max	-4.398	12	19.778	3	41.534	1	0	2	.005	5	.207	3
526		min	-69.622	1	-41.835	2	2.21	12	0	3	-.07	1	-.447	2
527	17	max	-4.398	12	67.792	3	74.112	1	0	2	.014	5	.174	3
528		min	-69.622	1	-146.279	2	3.637	12	0	3	-.027	1	-.376	2
529	18	max	-4.398	12	115.806	3	106.69	1	0	2	.041	1	.105	3
530		min	-69.622	1	-250.724	2	5.063	12	0	3	.003	12	-.227	2
531	19	max	-4.398	12	163.821	3	139.268	1	0	2	.133	1	0	2
532		min	-69.622	1	-355.168	2	6.49	12	0	3	.007	12	0	3
533	M15	1	max	0	1.847	1	.049	3	0	1	0	1	0	1
534		min	-63.965	3	0	2	-.039	1	0	3	0	3	0	1
535	2	max	0	2	1.642	1	.049	3	0	1	0	1	0	2
536		min	-64.04	3	0	2	-.039	1	0	3	0	3	0	1
537	3	max	0	2	1.437	1	.049	3	0	1	0	1	0	2
538		min	-64.116	3	0	2	-.039	1	0	3	0	3	-.001	1
539	4	max	0	2	1.231	1	.049	3	0	1	0	1	0	2
540		min	-64.191	3	0	2	-.039	1	0	3	0	3	-.002	1
541	5	max	0	2	1.026	1	.049	3	0	1	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-64.267	3	0	2	-.039	1	0	3	0	3	-.002	1
543		6	max	0	2	.821	1	.049	3	0	1	0	1	0	2
544			min	-64.343	3	0	2	-.039	1	0	3	0	3	-.003	1
545		7	max	0	2	.616	1	.049	3	0	1	0	3	0	2
546			min	-64.418	3	0	2	-.039	1	0	3	0	1	-.003	1
547		8	max	0	2	.41	1	.049	3	0	1	0	3	0	2
548			min	-64.494	3	0	2	-.039	1	0	3	0	1	-.003	1
549		9	max	0	2	.205	1	.049	3	0	1	0	3	0	2
550			min	-64.569	3	0	2	-.039	1	0	3	0	1	-.003	1
551		10	max	0	2	0	1	.049	3	0	1	0	3	0	2
552			min	-64.645	3	0	1	-.039	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.049	3	0	1	0	3	0	2
554			min	-64.72	3	-.205	1	-.039	1	0	3	0	1	-.003	1
555		12	max	0	2	0	2	.049	3	0	1	0	3	0	2
556			min	-64.796	3	-.41	1	-.039	1	0	3	0	1	-.003	1
557		13	max	0	2	0	2	.049	3	0	1	0	3	0	2
558			min	-64.871	3	-.616	1	-.039	1	0	3	0	1	-.003	1
559		14	max	0	2	0	2	.049	3	0	1	0	3	0	2
560			min	-64.947	3	-.821	1	-.039	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.049	3	0	1	0	3	0	2
562			min	-65.022	3	-1.026	1	-.039	1	0	3	0	1	-.002	1
563		16	max	0	2	0	2	.049	3	0	1	0	3	0	2
564			min	-65.098	3	-1.231	1	-.039	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.049	3	0	1	0	3	0	2
566			min	-65.173	3	-1.437	1	-.039	1	0	3	0	1	-.001	1
567		18	max	0	2	0	2	.049	3	0	1	0	3	0	2
568			min	-65.249	3	-1.642	1	-.039	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.049	3	0	1	0	3	0	1
570			min	-65.324	3	-1.847	1	-.039	1	0	3	0	1	0	1
571	M16A	1	max	-.859	10	3.146	4	.32	4	0	3	0	3	0	1
572			min	-234.383	4	.924	12	-.02	3	0	2	0	4	0	1
573		2	max	-.775	10	2.796	4	.288	4	0	3	0	3	0	12
574			min	-234.429	4	.821	12	-.02	3	0	2	0	4	-.001	4
575		3	max	-.691	10	2.447	4	.255	4	0	3	0	3	0	12
576			min	-234.475	4	.718	12	-.02	3	0	2	0	4	-.002	4
577		4	max	-.607	10	2.097	4	.223	4	0	3	0	3	0	12
578			min	-234.521	4	.616	12	-.02	3	0	2	0	4	-.003	4
579		5	max	-.523	10	1.748	4	.191	4	0	3	0	3	-.001	12
580			min	-234.568	4	.513	12	-.02	3	0	2	0	1	-.004	4
581		6	max	-.439	10	1.398	4	.159	4	0	3	0	5	-.001	12
582			min	-234.614	4	.41	12	-.02	3	0	2	0	1	-.005	4
583		7	max	-.355	10	1.049	4	.126	4	0	3	0	5	-.002	12
584			min	-234.66	4	.308	12	-.02	3	0	2	0	1	-.005	4
585		8	max	-.271	10	.699	4	.094	4	0	3	0	5	-.002	12
586			min	-234.706	4	.205	12	-.02	3	0	2	0	1	-.006	4
587		9	max	-.187	10	.35	4	.062	4	0	3	0	5	-.002	12
588			min	-234.753	4	.103	12	-.02	3	0	2	0	1	-.006	4
589		10	max	-.103	10	0	1	.029	4	0	3	0	5	-.002	12
590			min	-234.799	4	0	1	-.02	3	0	2	0	1	-.006	4
591		11	max	-.019	10	-.103	12	.023	1	0	3	0	5	-.002	12
592			min	-234.845	4	-.35	4	-.02	3	0	2	0	1	-.006	4
593		12	max	.064	10	-.205	12	.023	1	0	3	0	5	-.002	12
594			min	-234.891	4	-.699	4	-.039	5	0	2	0	1	-.006	4
595		13	max	.148	10	-.308	12	.023	1	0	3	0	5	-.002	12
596			min	-234.938	4	-1.049	4	-.071	5	0	2	0	3	-.005	4
597		14	max	.232	10	-.41	12	.023	1	0	3	0	4	-.001	12
598			min	-234.984	4	-1.398	4	-.103	5	0	2	0	3	-.005	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.316	10	-.513	12	.023	1	0	3	0	4	-.001	12
600		min	-235.03	4	-1.748	4	-.136	5	0	2	0	3	-.004	4
601	16	max	.4	10	-.616	12	.023	1	0	3	0	4	0	12
602		min	-235.076	4	-2.097	4	-.168	5	0	2	0	3	-.003	4
603	17	max	.484	10	-.718	12	.023	1	0	3	0	1	0	12
604		min	-235.123	4	-2.447	4	-.2	5	0	2	0	5	-.002	4
605	18	max	.568	10	-.821	12	.023	1	0	3	0	1	0	12
606		min	-235.169	4	-2.796	4	-.232	5	0	2	0	5	-.001	4
607	19	max	.652	10	-.924	12	.023	1	0	3	0	1	0	1
608		min	-235.215	4	-3.146	4	-.265	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.003	1	.01	2	.013	1	1.703e-3	5	NC	3	NC	3
2			min	-.004	3	-.011	3	-.016	5	-1.12e-3	1	4083.109	2	3196.295	1
3		2	max	.002	1	.009	2	.012	1	1.725e-3	5	NC	3	NC	3
4			min	-.004	3	-.01	3	-.016	5	-1.071e-3	1	4470.284	2	3431.114	1
5		3	max	.002	1	.009	2	.011	1	1.747e-3	5	NC	1	NC	3
6			min	-.003	3	-.01	3	-.016	5	-1.022e-3	1	4933.423	2	3709.302	1
7		4	max	.002	1	.008	2	.011	1	1.77e-3	5	NC	1	NC	2
8			min	-.003	3	-.01	3	-.015	5	-9.724e-4	1	5491.327	2	4041.046	1
9		5	max	.002	1	.007	2	.01	1	1.792e-3	5	NC	1	NC	2
10			min	-.003	3	-.009	3	-.015	5	-9.231e-4	1	6169.194	2	4439.951	1
11		6	max	.002	1	.006	2	.009	1	1.814e-3	5	NC	1	NC	2
12			min	-.003	3	-.009	3	-.014	5	-8.738e-4	1	7001.351	2	4924.502	1
13		7	max	.002	1	.005	2	.008	1	1.836e-3	5	NC	1	NC	2
14			min	-.003	3	-.008	3	-.014	5	-8.245e-4	1	8035.459	2	5520.326	1
15		8	max	.002	1	.005	2	.007	1	1.859e-3	5	NC	1	NC	2
16			min	-.002	3	-.008	3	-.013	5	-7.752e-4	1	9339.174	2	6263.785	1
17		9	max	.001	1	.004	2	.006	1	1.881e-3	5	NC	1	NC	2
18			min	-.002	3	-.007	3	-.012	5	-7.259e-4	1	NC	1	7207.921	1
19		10	max	.001	1	.003	2	.005	1	1.903e-3	5	NC	1	NC	2
20			min	-.002	3	-.007	3	-.012	5	-6.766e-4	1	NC	1	8432.664	1
21		11	max	.001	1	.003	2	.004	1	1.925e-3	5	NC	1	NC	1
22			min	-.002	3	-.006	3	-.011	5	-6.273e-4	1	NC	1	NC	1
23		12	max	.001	1	.002	2	.003	1	1.948e-3	5	NC	1	NC	1
24			min	-.001	3	-.005	3	-.01	5	-5.78e-4	1	NC	1	NC	1
25	13	max	0	1	.002	2	.003	1	1.97e-3	5	NC	1	NC	1	
26		min	-.001	3	-.005	3	-.009	5	-5.287e-4	1	NC	1	NC	1	
27	14	max	0	1	.001	2	.002	1	1.992e-3	5	NC	1	NC	1	
28		min	-.001	3	-.004	3	-.007	5	-4.794e-4	1	NC	1	NC	1	
29	15	max	0	1	0	2	.001	1	2.015e-3	5	NC	1	NC	1	
30		min	0	3	-.003	3	-.006	5	-4.301e-4	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	2.037e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.005	5	-3.808e-4	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	2.059e-3	5	NC	1	NC	1	
34		min	0	3	-.002	3	-.003	5	-3.316e-4	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	2.081e-3	5	NC	1	NC	1	
36		min	0	3	0	3	-.002	5	-2.823e-4	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	2.104e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-2.33e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.116e-4	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-1.006e-3	5	NC	1	NC	1
41		2	max	0	3	0	2	.005	5	1.349e-4	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-1.022e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.01	5	1.582e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.037e-3	5	NC	1	9633.166	14
45		4	max	0	3	0	2	.015	5	1.815e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-1.053e-3	5	NC	1	6314.433	14
47		5	max	0	3	0	2	.02	5	2.048e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	-1.068e-3	5	NC	1	4667.904	14
49		6	max	0	3	0	2	.025	5	2.282e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-1.084e-3	5	NC	1	3688.928	14
51		7	max	0	3	0	2	.03	4	2.515e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.099e-3	5	NC	1	3042.793	14
53		8	max	0	3	0	2	.035	4	2.748e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.115e-3	5	NC	1	2586.165	14
55		9	max	.001	3	.001	2	.04	4	2.981e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	1	-1.13e-3	5	NC	1	2247.454	14
57		10	max	.001	3	.002	2	.045	4	3.214e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-1.146e-3	5	NC	1	1986.942	14
59		11	max	.001	3	.002	2	.05	4	3.447e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-1.161e-3	5	NC	1	1780.834	14
61		12	max	.001	3	.003	2	.055	4	3.681e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-1.177e-3	5	NC	1	1614.016	14
63		13	max	.002	3	.004	2	.06	4	3.914e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	12	-1.192e-3	5	NC	1	1476.427	14
65		14	max	.002	3	.004	2	.064	4	4.147e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	12	-1.208e-3	5	NC	1	1361.115	14
67		15	max	.002	3	.005	2	.069	4	4.38e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	12	-1.223e-3	5	8822.132	2	1263.12	14
69		16	max	.002	3	.006	2	.074	4	4.613e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	12	-1.239e-3	5	7462.98	2	1178.812	14
71		17	max	.002	3	.007	2	.078	4	4.846e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	12	-1.254e-3	5	6414.837	2	1105.469	14
73		18	max	.002	3	.008	2	.083	4	5.08e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	12	-1.27e-3	5	5596.631	2	1041.01	14
75		19	max	.002	3	.009	2	.087	4	5.313e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	12	-1.285e-3	5	4951.791	2	983.817	14
77	M4	1	max	.002	1	.012	2	0	12	7.036e-3	5	NC	1	NC	3
78			min	0	15	-.011	3	-.092	4	-8.638e-4	1	NC	1	210.456	4
79		2	max	.002	1	.012	2	0	12	7.036e-3	5	NC	1	NC	3
80			min	0	15	-.01	3	-.084	4	-8.638e-4	1	NC	1	229.422	4
81		3	max	.002	1	.011	2	0	12	7.036e-3	5	NC	1	NC	2
82			min	0	15	-.01	3	-.077	4	-8.638e-4	1	NC	1	251.998	4
83		4	max	.001	1	.01	2	0	12	7.036e-3	5	NC	1	NC	2
84			min	0	15	-.009	3	-.069	4	-8.638e-4	1	NC	1	279.132	4
85		5	max	.001	1	.01	2	0	12	7.036e-3	5	NC	1	NC	2
86			min	0	15	-.008	3	-.062	4	-8.638e-4	1	NC	1	312.12	4
87		6	max	.001	1	.009	2	0	12	7.036e-3	5	NC	1	NC	2
88			min	0	15	-.008	3	-.055	4	-8.638e-4	1	NC	1	352.761	4
89		7	max	.001	1	.008	2	0	12	7.036e-3	5	NC	1	NC	2
90			min	0	15	-.007	3	-.048	4	-8.638e-4	1	NC	1	403.621	4
91		8	max	.001	1	.007	2	0	12	7.036e-3	5	NC	1	NC	2
92			min	0	15	-.007	3	-.041	4	-8.638e-4	1	NC	1	468.455	4
93		9	max	0	1	.007	2	0	12	7.036e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.035	4	-8.638e-4	1	NC	1	552.941	4
95		10	max	0	1	.006	2	0	12	7.036e-3	5	NC	1	NC	1
96			min	0	15	-.005	3	-.029	4	-8.638e-4	1	NC	1	666.007	4
97		11	max	0	1	.005	2	0	12	7.036e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.024	4	-8.638e-4	1	NC	1	822.359	4
99		12	max	0	1	.005	2	0	12	7.036e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.018	4	-8.638e-4	1	NC	1	1047.643	4
101		max	0	1	.004	2	0	12	7.036e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.014	4	-8.638e-4	1	NC	1	1389.97	4
103		max	0	1	.003	2	0	12	7.036e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.01	4	-8.638e-4	1	NC	1	1948.575	4
105		max	0	1	.003	2	0	12	7.036e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.007	4	-8.638e-4	1	NC	1	2956.598	4
107		max	0	1	.002	2	0	12	7.036e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.004	4	-8.638e-4	1	NC	1	5076.805	4
109		max	0	1	.001	2	0	12	7.036e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.002	4	-8.638e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	12	7.036e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-8.638e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	7.036e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-8.638e-4	1	NC	1	NC	1
115	M6	max	.008	1	.037	2	.005	1	1.865e-3	4	NC	3	NC	2
116		min	-.012	3	-.035	3	-.016	5	2.533e-7	10	1143.055	2	8850.968	1
117		max	.008	1	.035	2	.004	1	1.884e-3	4	NC	3	NC	2
118		min	-.011	3	-.033	3	-.016	5	-3.301e-7	10	1223.77	2	9564.729	1
119		max	.007	1	.032	2	.004	1	1.903e-3	4	NC	3	NC	1
120		min	-.011	3	-.031	3	-.016	5	-1.889e-6	2	1316.335	2	NC	1
121		max	.007	1	.03	2	.004	1	1.922e-3	4	NC	3	NC	1
122		min	-.01	3	-.029	3	-.016	5	-4.212e-6	2	1423.112	2	NC	1
123		max	.007	1	.027	2	.003	1	1.941e-3	4	NC	3	NC	1
124		min	-.009	3	-.027	3	-.015	5	-6.534e-6	2	1547.143	2	NC	1
125		max	.006	1	.025	2	.003	1	1.96e-3	4	NC	3	NC	1
126		min	-.009	3	-.026	3	-.015	5	-8.857e-6	2	1692.407	2	NC	1
127		max	.006	1	.023	2	.003	1	1.979e-3	4	NC	3	NC	1
128		min	-.008	3	-.024	3	-.014	5	-1.118e-5	2	1864.212	2	NC	1
129		max	.005	1	.021	2	.002	1	1.998e-3	4	NC	3	NC	1
130		min	-.007	3	-.022	3	-.014	5	-1.35e-5	2	2069.793	2	NC	1
131		max	.005	1	.018	2	.002	1	2.017e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.013	5	-1.582e-5	2	2319.276	2	NC	1
133		max	.004	1	.016	2	.002	1	2.036e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.012	5	-1.815e-5	2	2627.279	2	9931.28	4
135		max	.004	1	.014	2	.001	1	2.055e-3	4	NC	3	NC	1
136		min	-.005	3	-.016	3	-.011	5	-2.047e-5	2	3015.711	2	9983.507	4
137		max	.003	1	.012	2	.001	1	2.074e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.01	5	-2.279e-5	2	3518.982	2	NC	1
139		max	.003	1	.01	2	0	1	2.093e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.009	5	-2.511e-5	2	4194.404	2	NC	1
141		max	.002	1	.008	2	0	1	2.112e-3	4	NC	3	NC	1
142		min	-.003	3	-.01	3	-.008	5	-2.778e-5	11	5145.083	2	NC	1
143		max	.002	1	.006	2	0	1	2.131e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.006	5	-3.16e-5	11	6577.155	2	NC	1
145		max	.001	1	.005	2	0	1	2.15e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.005	5	-3.613e-5	1	8971.465	2	NC	1
147		max	0	1	.003	2	0	1	2.169e-3	5	NC	1	NC	1
148		min	-.001	3	-.004	3	-.003	5	-4.537e-5	1	NC	1	NC	1
149		max	0	1	.002	2	0	1	2.189e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-5.462e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	2.21e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-6.387e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	3.02e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.057e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.005	5	2.668e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-1.058e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.011	5	2.316e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.061e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.016	5	1.964e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.064e-3	4	NC	1	NC	1
161		5	max	.002	3	.006	2	.021	5	1.612e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-1.066e-3	4	7635.773	2	NC	1
163		6	max	.002	3	.008	2	.027	5	2.515e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	-1.069e-3	4	6114.8	2	NC	1
165		7	max	.003	3	.009	2	.032	5	4.455e-5	3	NC	3	NC	1
166			min	-.003	2	-.012	3	0	1	-1.072e-3	4	5078.125	2	NC	1
167		8	max	.003	3	.011	2	.037	5	6.394e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	-.001	1	-1.074e-3	4	4320.701	2	NC	1
169		9	max	.003	3	.012	2	.042	5	8.333e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	-.001	1	-1.077e-3	4	3740.265	2	NC	1
171		10	max	.004	3	.014	2	.047	5	1.027e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-1.08e-3	4	3280.095	2	NC	1
173		11	max	.004	3	.016	2	.052	5	1.221e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.083e-3	4	2906.126	2	NC	1
175		12	max	.005	3	.018	2	.056	4	1.415e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.002	1	-1.085e-3	4	2596.582	2	NC	1
177		13	max	.005	3	.02	2	.061	4	1.609e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.088e-3	4	2336.809	2	NC	1
179		14	max	.005	3	.022	2	.065	4	1.803e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-1.091e-3	4	2116.515	2	NC	1
181		15	max	.006	3	.024	2	.07	4	1.997e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.002	1	-1.093e-3	4	1928.205	2	NC	1
183		16	max	.006	3	.026	2	.074	4	2.191e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.002	1	-1.096e-3	4	1766.259	2	NC	1
185		17	max	.007	3	.028	2	.079	4	2.385e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.002	1	-1.099e-3	4	1626.354	2	NC	1
187		18	max	.007	3	.031	2	.083	4	2.579e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-1.101e-3	4	1505.101	2	NC	1
189		19	max	.008	3	.033	2	.087	4	2.773e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-1.104e-3	4	1399.8	2	NC	1
191	M8	1	max	.004	1	.043	2	.002	1	6.859e-3	4	NC	1	NC	2
192			min	0	15	-.034	3	-.092	4	-2.223e-4	3	NC	1	210.35	4
193		2	max	.004	1	.041	2	.002	1	6.859e-3	4	NC	1	NC	2
194			min	0	15	-.032	3	-.084	4	-2.223e-4	3	NC	1	229.306	4
195		3	max	.004	1	.038	2	.002	1	6.859e-3	4	NC	1	NC	2
196			min	0	15	-.03	3	-.077	4	-2.223e-4	3	NC	1	251.869	4
197		4	max	.004	1	.036	2	.002	1	6.859e-3	4	NC	1	NC	1
198			min	0	15	-.029	3	-.069	4	-2.223e-4	3	NC	1	278.989	4
199		5	max	.003	1	.033	2	.002	1	6.859e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.062	4	-2.223e-4	3	NC	1	311.958	4
201		6	max	.003	1	.031	2	.001	1	6.859e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.055	4	-2.223e-4	3	NC	1	352.577	4
203		7	max	.003	1	.029	2	.001	1	6.859e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.048	4	-2.223e-4	3	NC	1	403.41	4
205		8	max	.003	1	.026	2	.001	1	6.859e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.041	4	-2.223e-4	3	NC	1	468.207	4
207		9	max	.002	1	.024	2	0	1	6.859e-3	4	NC	1	NC	1
208			min	0	15	-.019	3	-.035	4	-2.223e-4	3	NC	1	552.647	4
209		10	max	.002	1	.022	2	0	1	6.859e-3	4	NC	1	NC	1
210			min	0	15	-.017	3	-.029	4	-2.223e-4	3	NC	1	665.652	4
211		11	max	.002	1	.019	2	0	1	6.859e-3	4	NC	1	NC	1
212			min	0	15	-.015	3	-.024	4	-2.223e-4	3	NC	1	821.917	4
213		12	max	.002	1	.017	2	0	1	6.859e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.009	4	3.044e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.034e-3	4	NC	1	NC	1
273		4	max	0	3	0	2	.013	4	9.993e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.128e-3	4	NC	1	NC	1
275		5	max	0	3	0	2	.017	4	-7.182e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-1.222e-3	4	NC	1	NC	1
277		6	max	0	3	0	2	.021	5	-2.009e-5	12	NC	1	NC	1
278			min	0	2	-.005	3	-.001	3	-1.316e-3	4	NC	1	NC	1
279		7	max	0	3	0	2	.026	5	-2.892e-5	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-1.409e-3	4	NC	1	NC	1
281		8	max	0	3	0	2	.03	5	-3.261e-5	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	1	-1.503e-3	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.034	5	-3.629e-5	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	1	-1.597e-3	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.039	5	-3.997e-5	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.004	1	-1.691e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.043	5	-4.365e-5	10	NC	1	NC	2
288			min	-.001	2	-.008	3	-.005	1	-1.784e-3	4	NC	1	9633.364	1
289		12	max	.001	3	.003	2	.047	5	-4.733e-5	10	NC	1	NC	2
290			min	-.001	2	-.008	3	-.006	1	-1.878e-3	4	NC	1	8003.542	1
291		13	max	.002	3	.004	2	.051	5	-5.102e-5	10	NC	1	NC	2
292			min	-.002	2	-.008	3	-.007	1	-1.972e-3	4	NC	1	6802.376	1
293		14	max	.002	3	.004	2	.055	5	-5.47e-5	10	NC	1	NC	2
294			min	-.002	2	-.009	3	-.008	1	-2.066e-3	4	NC	1	5891.496	1
295		15	max	.002	3	.005	2	.06	5	-5.838e-5	10	NC	1	NC	2
296			min	-.002	2	-.009	3	-.009	1	-2.159e-3	4	8834.619	2	5184.763	1
297		16	max	.002	3	.006	2	.064	5	-6.206e-5	10	NC	1	NC	2
298			min	-.002	2	-.009	3	-.01	1	-2.253e-3	4	7472.624	2	4626.214	1
299		17	max	.002	3	.007	2	.068	5	-6.574e-5	10	NC	1	NC	2
300			min	-.002	2	-.009	3	-.011	1	-2.347e-3	4	6422.487	2	4178.191	1
301		18	max	.002	3	.008	2	.072	5	-6.942e-5	10	NC	1	NC	2
302			min	-.002	2	-.009	3	-.012	1	-2.441e-3	4	5602.855	2	3814.603	1
303		19	max	.002	3	.009	2	.076	5	-7.311e-5	10	NC	3	NC	3
304			min	-.002	2	-.009	3	-.013	1	-2.535e-3	4	4956.978	2	3516.931	1
305	M12	1	max	.002	1	.012	2	.011	1	8.221e-3	4	NC	1	NC	3
306			min	0	15	-.011	3	-.084	5	8.225e-5	10	NC	1	229.699	5
307		2	max	.002	1	.012	2	.01	1	8.221e-3	4	NC	1	NC	3
308			min	0	15	-.01	3	-.077	5	8.225e-5	10	NC	1	250.394	5
309		3	max	.002	1	.011	2	.009	1	8.221e-3	4	NC	1	NC	3
310			min	0	15	-.01	3	-.07	5	8.225e-5	10	NC	1	275.027	5
311		4	max	.001	1	.01	2	.008	1	8.221e-3	4	NC	1	NC	3
312			min	0	15	-.009	3	-.063	5	8.225e-5	10	NC	1	304.632	5
313		5	max	.001	1	.009	2	.007	1	8.221e-3	4	NC	1	NC	3
314			min	0	15	-.008	3	-.057	5	8.225e-5	10	NC	1	340.625	5
315		6	max	.001	1	.009	2	.006	1	8.221e-3	4	NC	1	NC	3
316			min	0	15	-.008	3	-.05	5	8.225e-5	10	NC	1	384.967	5
317		7	max	.001	1	.008	2	.006	1	8.221e-3	4	NC	1	NC	3
318			min	0	15	-.007	3	-.044	5	8.225e-5	10	NC	1	440.458	5
319		8	max	.001	1	.007	2	.005	1	8.221e-3	4	NC	1	NC	2
320			min	0	15	-.007	3	-.038	5	8.225e-5	10	NC	1	511.194	5
321		9	max	0	1	.007	2	.004	1	8.221e-3	4	NC	1	NC	2
322			min	0	15	-.006	3	-.032	5	8.225e-5	10	NC	1	603.37	5
323		10	max	0	1	.006	2	.003	1	8.221e-3	4	NC	1	NC	2
324			min	0	15	-.005	3	-.027	5	8.225e-5	10	NC	1	726.726	5
325		11	max	0	1	.005	2	.003	1	8.221e-3	4	NC	1	NC	2
326			min	0	15	-.005	3	-.022	5	8.225e-5	10	NC	1	897.304	5
327		12	max	0	1	.005	2	.002	1	8.221e-3	4	NC	1	NC	2



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Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	15	-.004	3	-.017	5	8.225e-5	10	NC	1	1143.083	5
329		13	max	0	1	.004	2	.002	1	8.221e-3	4	NC	1	NC	1
330			min	0	15	-.004	3	-.013	5	8.225e-5	10	NC	1	1516.546	5
331		14	max	0	1	.003	2	.001	1	8.221e-3	4	NC	1	NC	1
332			min	0	15	-.003	3	-.009	5	8.225e-5	10	NC	1	2125.95	5
333		15	max	0	1	.003	2	0	1	8.221e-3	4	NC	1	NC	1
334			min	0	15	-.002	3	-.006	5	8.225e-5	10	NC	1	3225.622	5
335		16	max	0	1	.002	2	0	1	8.221e-3	4	NC	1	NC	1
336			min	0	15	-.002	3	-.003	5	8.225e-5	10	NC	1	5538.556	5
337		17	max	0	1	.001	2	0	1	8.221e-3	4	NC	1	NC	1
338			min	0	15	-.001	3	-.002	5	8.225e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	8.221e-3	4	NC	1	NC	1
340			min	0	15	0	3	0	5	8.225e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	8.221e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	8.225e-5	10	NC	1	NC	1
343	M1	1	max	.009	3	.026	3	.009	5	1.55e-2	1	NC	1	NC	1
344			min	-.009	2	-.023	2	-.005	1	-2.113e-2	3	NC	1	NC	1
345		2	max	.009	3	.016	3	.012	5	7.425e-3	2	NC	4	NC	2
346			min	-.009	2	-.014	2	-.01	1	-1.047e-2	3	4999.263	2	8995.44	1
347		3	max	.009	3	.007	3	.016	5	6.187e-4	5	NC	4	NC	2
348			min	-.009	2	-.005	2	-.013	1	-6.829e-4	1	2565.875	2	5459.766	1
349		4	max	.009	3	.003	1	.021	5	6.377e-4	5	NC	4	NC	2
350			min	-.009	2	-.002	3	-.015	1	-5.913e-4	1	1794.669	2	3972.562	5
351		5	max	.009	3	.01	2	.025	5	6.568e-4	5	NC	4	NC	2
352			min	-.009	2	-.008	3	-.015	1	-4.997e-4	1	1422.065	2	2840.641	5
353		6	max	.009	3	.015	2	.03	5	6.758e-4	5	NC	4	NC	2
354			min	-.009	2	-.014	3	-.014	1	-4.082e-4	1	1209.938	2	2180.736	5
355		7	max	.009	3	.02	2	.036	5	6.949e-4	5	NC	5	NC	2
356			min	-.009	2	-.018	3	-.013	1	-3.166e-4	1	1080.054	2	1753.883	5
357		8	max	.009	3	.024	2	.041	5	7.14e-4	5	NC	5	NC	2
358			min	-.009	2	-.021	3	-.01	1	-2.25e-4	1	999.845	2	1458.316	5
359		9	max	.009	3	.026	2	.047	5	7.33e-4	5	NC	5	NC	1
360			min	-.009	2	-.023	3	-.007	1	-1.334e-4	1	954.124	2	1242.218	4
361		10	max	.009	3	.027	2	.052	5	7.521e-4	5	NC	5	NC	1
362			min	-.009	2	-.023	3	-.004	1	-4.186e-5	1	935.961	2	1061.167	4
363		11	max	.009	3	.026	2	.058	4	7.852e-4	4	NC	5	NC	1
364			min	-.009	2	-.022	3	-.001	1	1.488e-5	10	943.468	2	925.568	4
365		12	max	.009	3	.025	2	.064	4	8.264e-4	4	NC	5	NC	2
366			min	-.009	2	-.021	3	0	10	2.421e-5	10	979.128	2	821.69	4
367		13	max	.009	3	.021	2	.071	4	8.676e-4	4	NC	4	NC	2
368			min	-.009	2	-.018	3	0	10	3.219e-5	12	1051.144	2	740.766	4
369		14	max	.009	3	.016	2	.077	4	9.088e-4	4	NC	4	NC	2
370			min	-.009	2	-.013	3	0	12	3.594e-5	12	1178.242	2	676.994	4
371		15	max	.009	3	.01	2	.082	4	9.5e-4	4	NC	4	NC	2
372			min	-.009	2	-.008	3	0	12	3.969e-5	12	1404.182	2	626.422	4
373		16	max	.009	3	.002	1	.087	4	1.322e-3	4	NC	4	NC	2
374			min	-.009	2	-.002	3	0	12	4.243e-5	12	1850.769	2	586.301	4
375		17	max	.009	3	.006	3	.092	4	9.58e-3	4	NC	4	NC	2
376			min	-.009	2	-.008	2	0	12	-1.63e-4	1	2657.803	1	554.721	4
377		18	max	.009	3	.014	3	.096	4	1.111e-2	2	NC	2	NC	2
378			min	-.009	2	-.019	2	0	10	-5.251e-3	3	5138.037	1	530.257	4
379		19	max	.009	3	.022	3	.099	4	2.249e-2	2	NC	1	NC	1
380			min	-.009	2	-.031	2	-.003	1	-1.063e-2	3	5933.943	2	512.547	4
381	M5	1	max	.03	3	.086	3	.008	5	1.054e-5	4	NC	1	NC	1
382			min	-.033	2	-.078	2	-.006	1	4.16e-8	10	3584.04	3	NC	1
383		2	max	.03	3	.052	3	.012	5	3.136e-4	5	NC	5	NC	1
384			min	-.033	2	-.047	2	-.005	1	-5.782e-5	1	1494.373	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.021	3	.016	5	6.117e-4	5	NC	5	NC	1
386		min	-.033	2	-.017	2	-.005	1	-1.152e-4	1	766.545	2	NC	1
387	4	max	.03	3	.009	2	.021	5	6.394e-4	5	NC	5	NC	1
388		min	-.033	2	-.005	3	-.004	1	-1.105e-4	1	535.613	2	NC	1
389	5	max	.03	3	.032	2	.026	5	6.671e-4	5	NC	5	NC	1
390		min	-.033	2	-.027	3	-.004	1	-1.057e-4	1	423.995	2	NC	1
391	6	max	.029	3	.052	2	.032	5	6.948e-4	5	NC	5	NC	1
392		min	-.033	2	-.045	3	-.004	1	-1.01e-4	1	360.419	2	NC	1
393	7	max	.029	3	.067	2	.037	5	7.226e-4	5	NC	15	NC	1
394		min	-.033	2	-.059	3	-.004	1	-9.63e-5	1	321.463	2	NC	1
395	8	max	.029	3	.079	2	.043	5	7.503e-4	5	NC	15	NC	1
396		min	-.033	2	-.068	3	-.003	1	-9.158e-5	1	297.372	2	NC	1
397	9	max	.029	3	.087	2	.049	5	7.78e-4	5	NC	15	NC	1
398		min	-.033	2	-.074	3	-.003	1	-8.685e-5	1	283.595	2	NC	1
399	10	max	.029	3	.09	2	.055	5	8.057e-4	5	NC	15	NC	1
400		min	-.033	2	-.075	3	-.003	1	-8.213e-5	1	278.053	2	NC	1
401	11	max	.029	3	.089	2	.061	5	8.335e-4	5	NC	15	NC	1
402		min	-.033	2	-.073	3	-.003	1	-7.74e-5	1	280.178	2	NC	1
403	12	max	.029	3	.083	2	.067	5	8.612e-4	5	NC	15	NC	1
404		min	-.033	2	-.067	3	-.003	1	-7.268e-5	1	290.708	2	NC	1
405	13	max	.029	3	.072	2	.072	5	8.889e-4	5	NC	15	NC	1
406		min	-.033	2	-.057	3	-.003	1	-6.796e-5	1	312.098	2	NC	1
407	14	max	.029	3	.055	2	.078	5	9.166e-4	5	NC	5	NC	1
408		min	-.033	2	-.043	3	-.003	1	-6.323e-5	1	349.969	2	NC	1
409	15	max	.028	3	.034	2	.083	4	9.444e-4	5	NC	5	NC	1
410		min	-.033	2	-.026	3	-.002	1	-5.851e-5	1	417.498	2	NC	1
411	16	max	.028	3	.007	1	.087	4	1.302e-3	5	NC	5	NC	1
412		min	-.033	2	-.006	3	-.002	1	-6.055e-5	1	551.603	2	NC	1
413	17	max	.028	3	.019	3	.092	4	9.556e-3	4	NC	5	NC	1
414		min	-.033	2	-.027	2	-.002	1	-2.237e-4	1	869.3	3	NC	1
415	18	max	.028	3	.045	3	.096	4	4.901e-3	4	NC	5	NC	1
416		min	-.033	2	-.065	2	-.002	1	-1.144e-4	1	1704.93	3	NC	1
417	19	max	.028	3	.073	3	.099	4	2.509e-6	5	NC	3	NC	1
418		min	-.033	2	-.106	2	-.002	1	-4.663e-7	3	1716.184	2	NC	1
419	M9	1	max	.009	.026	.007	.007	5	2.113e-2	3	NC	1	NC	1
420		min	-.009	2	-.023	2	-.006	1	-1.549e-2	1	NC	1	NC	1
421	2	max	.009	3	.016	.007	.007	5	1.044e-2	3	NC	4	NC	2
422		min	-.009	2	-.014	2	-.001	1	-7.534e-3	1	5001.009	2	9871.653	1
423	3	max	.009	3	.006	.007	.007	4	2.774e-4	1	NC	4	NC	2
424		min	-.009	2	-.005	2	0	3	-6.45e-5	3	2566.795	2	6073.019	1
425	4	max	.009	3	.003	.009	.009	4	2.e-4	1	NC	4	NC	2
426		min	-.009	2	-.002	3	-.001	3	-6.877e-5	3	1795.315	2	5098.868	1
427	5	max	.009	3	.01	.012	.012	4	1.225e-4	1	NC	4	NC	2
428		min	-.009	2	-.009	3	-.002	3	-7.305e-5	3	1422.561	2	4991.076	1
429	6	max	.009	3	.015	.015	.015	4	8.437e-5	4	NC	4	NC	2
430		min	-.009	2	-.014	3	-.003	3	-7.733e-5	3	1210.337	2	4839.668	4
431	7	max	.009	3	.02	.019	.019	4	9.87e-5	4	NC	4	NC	2
432		min	-.009	2	-.018	3	-.003	3	-8.16e-5	3	1080.386	2	3474.297	4
433	8	max	.009	3	.024	.023	.023	4	1.302e-4	5	NC	5	NC	1
434		min	-.009	2	-.021	3	-.004	3	-1.098e-4	1	1000.126	2	2623.417	4
435	9	max	.009	3	.026	.028	.028	5	1.616e-4	5	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	3	-1.872e-4	1	954.362	2	2058.034	4
437	10	max	.009	3	.027	.034	.034	5	1.931e-4	5	NC	5	NC	1
438		min	-.009	2	-.023	3	-.005	1	-2.646e-4	1	936.161	2	1663.306	4
439	11	max	.009	3	.026	.041	.041	5	2.245e-4	5	NC	5	NC	1
440		min	-.009	2	-.023	3	-.008	1	-3.421e-4	1	943.628	2	1376.721	4
441	12	max	.009	3	.025	.047	.047	5	2.56e-4	5	NC	5	NC	2



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Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.191	3	.087	1	6.974e-3	2	NC	5	NC	3
500		min	-.099	4	-.387	2	.004	10	-4.851e-3	3	455.082	2	1772.093	1
501	4	max	.002	1	.24	3	.131	1	8.178e-3	2	NC	5	NC	10
502		min	-.099	4	-.489	2	.007	10	-5.653e-3	3	354.088	2	1196.975	1
503	5	max	.002	1	.257	3	.152	1	9.381e-3	2	NC	5	NC	10
504		min	-.099	4	-.52	2	.008	10	-6.456e-3	3	331.336	2	1036.864	1
505	6	max	.002	1	.242	3	.143	1	1.058e-2	2	NC	5	NC	10
506		min	-.099	4	-.483	2	.005	10	-7.258e-3	3	358.828	2	1095.268	1
507	7	max	.002	1	.201	3	.108	1	1.179e-2	2	NC	5	NC	5
508		min	-.099	4	-.39	2	0	10	-8.06e-3	3	451.435	2	1442.849	1
509	8	max	.002	1	.146	3	.055	1	1.299e-2	2	NC	5	NC	2
510		min	-.099	4	-.268	2	-.009	10	-8.862e-3	3	682.704	2	2726.62	1
511	9	max	.002	1	.096	3	.031	3	1.42e-2	2	NC	4	NC	1
512		min	-.099	4	-.157	2	-.023	2	-9.665e-3	3	1291.232	2	7295.543	3
513	10	max	.002	1	.073	3	.028	3	1.54e-2	2	NC	4	NC	4
514		min	-.099	4	-.106	2	-.033	2	-1.047e-2	3	2173.572	2	6776.662	2
515	11	max	.003	1	.096	3	.028	3	1.42e-2	2	NC	4	NC	1
516		min	-.099	4	-.157	2	-.022	2	-9.664e-3	3	1291.232	2	8604.349	3
517	12	max	.003	1	.146	3	.054	1	1.299e-2	2	NC	5	NC	2
518		min	-.099	4	-.268	2	-.009	10	-8.86e-3	3	682.704	2	2761.48	1
519	13	max	.003	1	.201	3	.106	1	1.179e-2	2	NC	5	NC	5
520		min	-.099	4	-.39	2	0	10	-8.057e-3	3	451.435	2	1459.257	1
521	14	max	.003	1	.242	3	.142	1	1.059e-2	2	NC	5	NC	5
522		min	-.099	4	-.483	2	.005	15	-7.253e-3	3	358.828	2	1108.261	1
523	15	max	.003	1	.257	3	.15	1	9.383e-3	2	NC	5	NC	5
524		min	-.099	4	-.52	2	.001	15	-6.45e-3	3	331.336	2	1050.753	1
525	16	max	.003	1	.24	3	.129	1	8.179e-3	2	NC	5	NC	3
526		min	-.099	4	-.489	2	-.004	5	-5.646e-3	3	354.088	2	1216.358	1
527	17	max	.003	1	.191	3	.085	1	6.976e-3	2	NC	5	NC	3
528		min	-.099	4	-.387	2	-.008	5	-4.843e-3	3	455.082	2	1809.895	1
529	18	max	.003	1	.115	3	.033	1	5.772e-3	2	NC	5	NC	2
530		min	-.099	4	-.227	2	-.008	5	-4.04e-3	3	828.355	2	4309.978	1
531	19	max	.003	1	.022	3	.009	3	4.569e-3	2	NC	1	NC	1
532		min	-.099	4	-.031	2	-.009	2	-3.236e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.003e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.235e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.011	4	9.092e-4	3	NC	3	NC	1
536		min	0	5	-.011	1	0	3	-7.49e-4	5	7992.014	2	8271.858	4
537	3	max	0	3	0	15	.025	4	1.418e-3	3	NC	5	NC	1
538		min	-.002	5	-.023	1	-.004	3	-1.108e-3	2	4066.863	2	3680.672	4
539	4	max	0	3	0	15	.04	4	1.927e-3	3	NC	5	NC	9
540		min	-.002	5	-.033	1	-.008	3	-1.633e-3	2	2790.106	2	2308.85	4
541	5	max	0	3	0	15	.054	4	2.436e-3	3	NC	5	NC	9
542		min	-.003	5	-.042	1	-.013	3	-2.157e-3	2	2177.148	2	1698.286	4
543	6	max	0	3	0	15	.066	4	2.945e-3	3	NC	5	9977.038	9
544		min	-.004	5	-.05	1	-.019	3	-2.682e-3	2	1832.3	2	1378.514	4
545	7	max	0	3	0	15	.076	4	3.453e-3	3	NC	5	7922.298	9
546		min	-.005	5	-.056	1	-.025	3	-3.207e-3	2	1624.92	2	1201.328	4
547	8	max	0	3	0	15	.082	4	3.962e-3	3	NC	5	6611.605	9
548		min	-.006	5	-.061	1	-.031	3	-3.731e-3	2	1500.46	2	1108.32	4
549	9	max	0	3	0	15	.085	4	4.471e-3	3	NC	5	5746.038	9
550		min	-.007	5	-.064	1	-.036	3	-4.256e-3	2	1433.469	2	1074.833	4
551	10	max	0	3	.001	15	.084	4	4.98e-3	3	NC	5	5173.026	9
552		min	-.007	5	-.065	1	-.04	3	-4.781e-3	2	1412.276	2	1092.811	4
553	11	max	0	3	.001	15	.078	4	5.489e-3	3	NC	5	4811.976	9
554		min	-.008	5	-.064	1	-.043	3	-5.305e-3	2	1433.469	2	1166.03	4
555	12	max	0	3	.002	15	.07	4	5.998e-3	3	NC	5	4623.651	9





Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
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E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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