

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

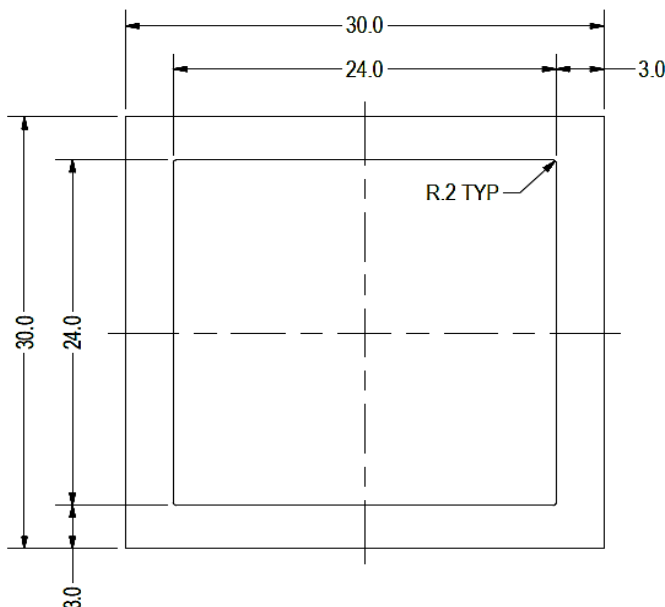
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

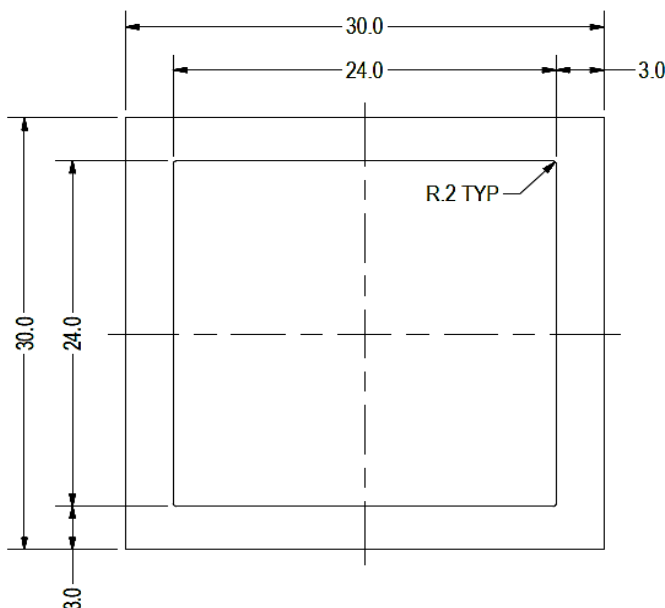
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.714 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.571 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	15%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.604 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.118 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 37 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

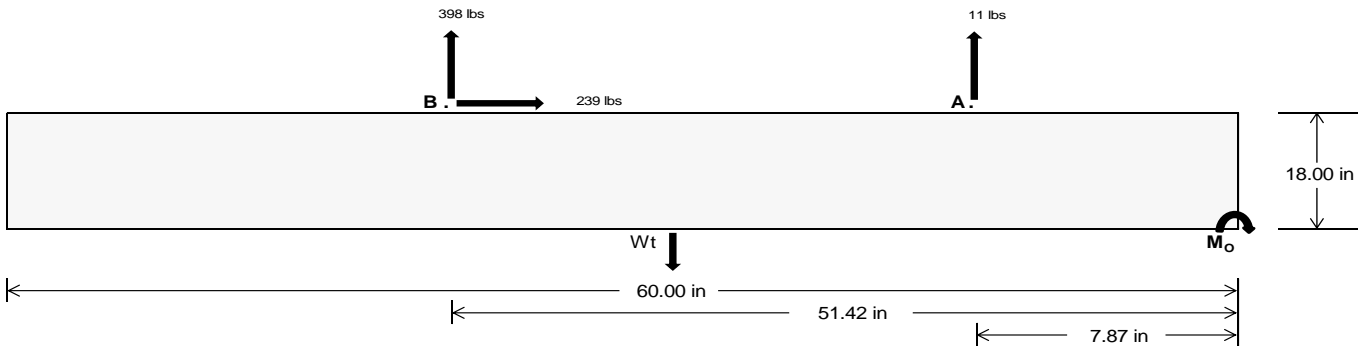
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>49.61</u>	<u>1659.31</u>	k
Compressive Load =	<u>927.67</u>	<u>1086.20</u>	k
Lateral Load =	<u>1.52</u>	<u>992.63</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24871.6$ in-lbs
Resisting Force Required = 829.05 lbs
S.F. = 1.67
Weight Required = 1381.76 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 238.57 lbs
Friction = 0.4
Weight Required = 596.43 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 238.57 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

		Ballast Width			
		21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$		1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	303 lbs	303 lbs	303 lbs	303 lbs	370 lbs	370 lbs	370 lbs	370 lbs	477 lbs	477 lbs	477 lbs	477 lbs	-22 lbs	-22 lbs	-22 lbs	-22 lbs
F_B	202 lbs	202 lbs	202 lbs	202 lbs	481 lbs	481 lbs	481 lbs	481 lbs	493 lbs	493 lbs	493 lbs	493 lbs	-797 lbs	-797 lbs	-797 lbs	-797 lbs
F_V	25 lbs	25 lbs	25 lbs	25 lbs	426 lbs	426 lbs	426 lbs	426 lbs	337 lbs	337 lbs	337 lbs	337 lbs	-477 lbs	-477 lbs	-477 lbs	-477 lbs
P_{total}	2409 lbs	2499 lbs	2590 lbs	2681 lbs	2754 lbs	2845 lbs	2936 lbs	3026 lbs	2873 lbs	2964 lbs	3054 lbs	3145 lbs	323 lbs	377 lbs	431 lbs	486 lbs
M	236 lbs-ft	236 lbs-ft	236 lbs-ft	236 lbs-ft	464 lbs-ft	464 lbs-ft	464 lbs-ft	464 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	665 lbs-ft	665 lbs-ft	665 lbs-ft	665 lbs-ft
e	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.06 ft	1.77 ft	1.54 ft	1.37 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	243.0 psf	241.8 psf	240.7 psf	239.8 psf	251.2 psf	249.6 psf	248.2 psf	246.9 psf	259.1 psf	257.2 psf	255.5 psf	253.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	307.6 psf	303.5 psf	299.8 psf	296.4 psf	378.4 psf	371.1 psf	364.4 psf	358.3 psf	397.6 psf	389.4 psf	381.9 psf	375.1 psf	281.4 psf	186.6 psf	156.8 psf	143.3 psf

Maximum Bearing Pressure = 398 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

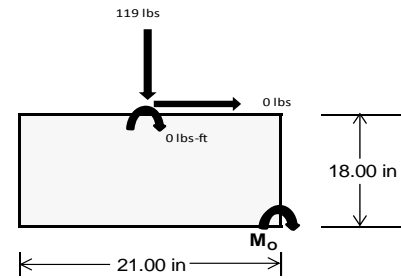
Overturning Check

$M_o = 0.0 \text{ ft-lbs}$
 Resisting Force Required = 0.00 lbs
 S.F. = 1.67
 Weight Required = 0.00 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	50 lbs	119 lbs	47 lbs	153 lbs	420 lbs	150 lbs	15 lbs	35 lbs	14 lbs
F_v	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2406 lbs	2475 lbs	2403 lbs	2396 lbs	2663 lbs	2393 lbs	704 lbs	724 lbs	703 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	274.9 sqft	282.8 sqft	274.6 sqft	273.5 sqft	304.2 sqft	273.3 sqft	80.4 sqft	82.7 sqft	80.3 sqft
f_{max}	275.1 psf	282.9 psf	274.7 psf	274.1 psf	304.5 psf	273.6 psf	80.4 psf	82.7 psf	80.3 psf



Maximum Bearing Pressure = 304 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.784 k
Allowable Uplift =	1.214 k
Utilization =	<u>65%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.052 k
Allowable Uplift =	1.116 k
Utilization =	<u>94%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.714 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

Diagonal Strut

Maximum Axial Load =	0.571 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>



Rear Strut

Maximum Axial Load =	1.042 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Bracing

Maximum Axial Load =	0.118 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.004 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$117.177$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$121.682$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.271 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &21.4323 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	222.505	2	271.273	2	.004	10	0	10	0	1	0	1
2		min	-260.982	3	-409.581	3	-.15	3	0	3	0	1	0	1
3	N7	max	.002	3	256.042	1	.036	10	0	10	0	1	0	1
4		min	-.12	2	3.388	12	-.497	1	0	1	0	1	0	1
5	N15	max	0	15	713.595	1	.114	9	0	9	0	1	0	1
6		min	-1.168	2	-38.16	3	-.69	3	-.001	3	0	1	0	1
7	N16	max	691.322	2	835.538	2	0	2	0	9	0	1	0	1
8		min	-763.564	3	-1276.394	3	-87.704	3	0	3	0	1	0	1
9	N23	max	.002	3	256.357	1	.579	1	0	1	0	1	0	1
10		min	-.12	2	3.847	12	-.036	10	0	10	0	1	0	1
11	N24	max	222.505	2	273.598	2	88.476	3	0	9	0	1	0	1
12		min	-261.649	3	-409.003	3	-.004	10	0	3	0	1	0	1
13	Totals:	max	1134.924	2	2520.141	2	0	3						
14		min	-1286.244	3	-2123.654	3	0	9						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	194.315	2	.656	4	.085	1	0	10	0	10	0	1
2			min	-350.604	3	.154	15	-.073	3	0	1	0	1	0	1
3		2	max	194.441	2	.605	4	.085	1	0	10	0	15	0	15
4			min	-350.509	3	.142	15	-.073	3	0	1	0	3	0	4
5		3	max	194.567	2	.554	4	.085	1	0	10	0	15	0	15
6			min	-350.415	3	.13	15	-.073	3	0	1	0	3	0	4
7		4	max	194.693	2	.502	4	.085	1	0	10	0	9	0	15
8			min	-350.321	3	.118	15	-.073	3	0	1	0	3	0	4
9		5	max	194.818	2	.451	4	.085	1	0	10	0	9	0	15
10			min	-350.226	3	.106	15	-.073	3	0	1	0	3	0	4
11		6	max	194.944	2	.4	4	.085	1	0	10	0	9	0	15
12			min	-350.132	3	.094	15	-.073	3	0	1	0	3	0	4
13		7	max	195.07	2	.349	4	.085	1	0	10	0	9	0	15
14			min	-350.037	3	.082	15	-.073	3	0	1	0	3	0	4
15		8	max	195.196	2	.298	4	.085	1	0	10	0	9	0	15
16			min	-349.943	3	.07	15	-.073	3	0	1	0	3	0	4
17		9	max	195.322	2	.247	4	.085	1	0	10	0	9	0	15
18			min	-349.849	3	.058	15	-.073	3	0	1	0	3	0	4
19		10	max	195.448	2	.196	4	.085	1	0	10	0	9	0	15
20			min	-349.754	3	.046	15	-.073	3	0	1	0	3	0	4
21		11	max	195.574	2	.144	4	.085	1	0	10	0	9	0	15
22			min	-349.66	3	.033	12	-.073	3	0	1	0	3	0	4
23		12	max	195.699	2	.103	2	.085	1	0	10	0	9	0	15
24			min	-349.565	3	.013	12	-.073	3	0	1	0	3	0	4
25		13	max	195.825	2	.063	2	.085	1	0	10	0	9	0	15
26			min	-349.471	3	-.013	3	-.073	3	0	1	0	3	0	4
27		14	max	195.951	2	.023	2	.085	1	0	10	0	9	0	15
28			min	-349.377	3	-.043	3	-.073	3	0	1	0	3	0	4
29		15	max	196.077	2	-.014	15	.085	1	0	10	0	9	0	15
30			min	-349.282	3	-.072	3	-.073	3	0	1	0	3	0	4
31		16	max	196.203	2	-.026	15	.085	1	0	10	0	1	0	15
32			min	-349.188	3	-.111	4	-.073	3	0	1	0	3	0	4
33		17	max	196.329	2	-.038	15	.085	1	0	10	0	1	0	15
34			min	-349.093	3	-.162	4	-.073	3	0	1	0	3	0	4
35		18	max	196.455	2	-.05	15	.085	1	0	10	0	1	0	15
36			min	-348.999	3	-.214	4	-.073	3	0	1	0	3	0	4
37		19	max	196.581	2	-.062	15	.085	1	0	10	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-348.905	3	-.265	4	-.073	3	0	1	0	3	0	4
39	M3	1	max	183.61	2	1.759	4	.006	10	0	10	0	1	4
40		min	-173.449	3	.414	15	-.122	1	0	1	0	10	0	15
41		2	max	183.541	2	1.582	4	.006	10	0	10	0	1	2
42		min	-173.501	3	.372	15	-.122	1	0	1	0	10	0	12
43		3	max	183.471	2	1.405	4	.006	10	0	10	0	1	2
44		min	-173.553	3	.33	15	-.122	1	0	1	0	10	0	3
45		4	max	183.402	2	1.228	4	.006	10	0	10	0	1	15
46		min	-173.605	3	.289	15	-.122	1	0	1	0	10	0	4
47		5	max	183.333	2	1.052	4	.006	10	0	10	0	1	15
48		min	-173.657	3	.247	15	-.122	1	0	1	0	10	0	4
49		6	max	183.263	2	.875	4	.006	10	0	10	0	1	15
50		min	-173.709	3	.206	15	-.122	1	0	1	0	10	0	4
51		7	max	183.194	2	.698	4	.006	10	0	10	0	1	15
52		min	-173.761	3	.164	15	-.122	1	0	1	0	10	0	4
53		8	max	183.125	2	.521	4	.006	10	0	10	0	1	15
54		min	-173.813	3	.123	15	-.122	1	0	1	0	10	-.001	4
55		9	max	183.055	2	.344	4	.006	10	0	10	0	1	15
56		min	-173.865	3	.081	15	-.122	1	0	1	0	10	-.001	4
57		10	max	182.986	2	.167	4	.006	10	0	10	0	1	15
58		min	-173.917	3	.039	15	-.122	1	0	1	0	10	-.001	4
59		11	max	182.917	2	.018	2	.006	10	0	10	0	1	15
60		min	-173.969	3	-.037	3	-.122	1	0	1	0	10	-.001	4
61		12	max	182.847	2	-.044	15	.006	10	0	10	0	1	15
62		min	-174.021	3	-.186	4	-.122	1	0	1	0	10	-.001	4
63		13	max	182.778	2	-.085	15	.006	10	0	10	0	1	15
64		min	-174.073	3	-.363	4	-.122	1	0	1	0	10	-.001	4
65		14	max	182.709	2	-.127	15	.006	10	0	10	0	1	15
66		min	-174.125	3	-.54	4	-.122	1	0	1	0	10	-.001	4
67		15	max	182.639	2	-.168	15	.006	10	0	10	0	9	15
68		min	-174.177	3	-.717	4	-.122	1	0	1	0	10	0	4
69		16	max	182.57	2	-.21	15	.006	10	0	10	0	9	15
70		min	-174.229	3	-.894	4	-.122	1	0	1	0	1	0	4
71		17	max	182.501	2	-.251	15	.006	10	0	10	0	10	15
72		min	-174.281	3	-1.07	4	-.122	1	0	1	0	1	0	4
73		18	max	182.431	2	-.293	15	.006	10	0	10	0	10	15
74		min	-174.333	3	-1.247	4	-.122	1	0	1	0	1	0	4
75		19	max	182.362	2	-.335	15	.006	10	0	10	0	10	1
76		min	-174.385	3	-1.424	4	-.122	1	0	1	0	1	0	1
77	M4	1	max	254.877	1	0	1	.037	10	0	1	0	3	1
78		min	2.806	12	0	1	-.519	1	0	1	0	2	0	1
79		2	max	254.942	1	0	1	.037	10	0	1	0	15	1
80		min	2.838	12	0	1	-.519	1	0	1	0	1	0	1
81		3	max	255.007	1	0	1	.037	10	0	1	0	10	1
82		min	2.87	12	0	1	-.519	1	0	1	0	1	0	1
83		4	max	255.072	1	0	1	.037	10	0	1	0	10	1
84		min	2.903	12	0	1	-.519	1	0	1	0	1	0	1
85		5	max	255.136	1	0	1	.037	10	0	1	0	10	1
86		min	2.935	12	0	1	-.519	1	0	1	0	1	0	1
87		6	max	255.201	1	0	1	.037	10	0	1	0	10	1
88		min	2.967	12	0	1	-.519	1	0	1	0	1	0	1
89		7	max	255.266	1	0	1	.037	10	0	1	0	10	1
90		min	3	12	0	1	-.519	1	0	1	0	1	0	1
91		8	max	255.33	1	0	1	.037	10	0	1	0	10	1
92		min	3.032	12	0	1	-.519	1	0	1	0	1	0	1
93		9	max	255.395	1	0	1	.037	10	0	1	0	10	1
94		min	3.064	12	0	1	-.519	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	255.46	1	0	1	.037	10	0	1	0	10	0	1
96		min	3.097	12	0	1	-.519	1	0	1	0	1	0	1
97	11	max	255.525	1	0	1	.037	10	0	1	0	10	0	1
98		min	3.129	12	0	1	-.519	1	0	1	0	1	0	1
99	12	max	255.589	1	0	1	.037	10	0	1	0	10	0	1
100		min	3.161	12	0	1	-.519	1	0	1	0	1	0	1
101	13	max	255.654	1	0	1	.037	10	0	1	0	10	0	1
102		min	3.194	12	0	1	-.519	1	0	1	0	1	0	1
103	14	max	255.719	1	0	1	.037	10	0	1	0	10	0	1
104		min	3.226	12	0	1	-.519	1	0	1	0	1	0	1
105	15	max	255.783	1	0	1	.037	10	0	1	0	10	0	1
106		min	3.258	12	0	1	-.519	1	0	1	0	1	0	1
107	16	max	255.848	1	0	1	.037	10	0	1	0	10	0	1
108		min	3.291	12	0	1	-.519	1	0	1	0	1	0	1
109	17	max	255.913	1	0	1	.037	10	0	1	0	10	0	1
110		min	3.323	12	0	1	-.519	1	0	1	0	1	0	1
111	18	max	255.977	1	0	1	.037	10	0	1	0	10	0	1
112		min	3.356	12	0	1	-.519	1	0	1	0	1	0	1
113	19	max	256.042	1	0	1	.037	10	0	1	0	10	0	1
114		min	3.388	12	0	1	-.519	1	0	1	0	1	0	1
115	M6	1	max	601.557	2	.656	.017	9	0	3	0	3	0	1
116		min	-1042.315	3	.154	15	-.277	3	0	2	0	2	0	1
117	2	max	601.683	2	.604	4	.017	9	0	3	0	3	0	15
118		min	-1042.221	3	.142	15	-.277	3	0	2	0	2	0	4
119	3	max	601.809	2	.553	4	.017	9	0	3	0	3	0	15
120		min	-1042.126	3	.13	15	-.277	3	0	2	0	2	0	4
121	4	max	601.935	2	.502	4	.017	9	0	3	0	3	0	15
122		min	-1042.032	3	.118	15	-.277	3	0	2	0	2	0	4
123	5	max	602.061	2	.451	4	.017	9	0	3	0	3	0	15
124		min	-1041.937	3	.106	15	-.277	3	0	2	0	2	0	4
125	6	max	602.187	2	.402	2	.017	9	0	3	0	3	0	15
126		min	-1041.843	3	.086	12	-.277	3	0	2	0	2	0	4
127	7	max	602.312	2	.363	2	.017	9	0	3	0	9	0	15
128		min	-1041.749	3	.067	12	-.277	3	0	2	0	3	0	4
129	8	max	602.438	2	.323	2	.017	9	0	3	0	9	0	15
130		min	-1041.654	3	.047	12	-.277	3	0	2	0	3	0	4
131	9	max	602.564	2	.283	2	.017	9	0	3	0	9	0	15
132		min	-1041.56	3	.027	12	-.277	3	0	2	0	3	0	4
133	10	max	602.69	2	.243	2	.017	9	0	3	0	9	0	12
134		min	-1041.465	3	.003	3	-.277	3	0	2	0	3	0	4
135	11	max	602.816	2	.203	2	.017	9	0	3	0	9	0	12
136		min	-1041.371	3	-.027	3	-.277	3	0	2	0	3	0	2
137	12	max	602.942	2	.163	2	.017	9	0	3	0	9	0	12
138		min	-1041.277	3	-.057	3	-.277	3	0	2	0	3	0	2
139	13	max	603.068	2	.123	2	.017	9	0	3	0	9	0	12
140		min	-1041.182	3	-.086	3	-.277	3	0	2	0	3	0	2
141	14	max	603.194	2	.084	2	.017	9	0	3	0	9	0	12
142		min	-1041.088	3	-.116	3	-.277	3	0	2	0	3	0	2
143	15	max	603.319	2	.044	2	.017	9	0	3	0	9	0	12
144		min	-1040.993	3	-.146	3	-.277	3	0	2	0	3	0	2
145	16	max	603.445	2	.004	2	.017	9	0	3	0	9	0	12
146		min	-1040.899	3	-.176	3	-.277	3	0	2	0	3	0	2
147	17	max	603.571	2	-.036	2	.017	9	0	3	0	9	0	12
148		min	-1040.805	3	-.206	3	-.277	3	0	2	0	3	0	2
149	18	max	603.697	2	-.05	15	.017	9	0	3	0	9	0	12
150		min	-1040.71	3	-.236	3	-.277	3	0	2	0	3	0	2
151	19	max	603.823	2	-.062	15	.017	9	0	3	0	9	0	3



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152			min	-1040.616	3	-.266	3	-.277	3	0	2	0	3	0	2
153	M7	1	max	571.344	2	1.761	4	.038	3	0	9	0	1	0	2
154			min	-475.711	3	.414	15	-.015	1	0	3	0	3	0	3
155		2	max	571.274	2	1.584	4	.038	3	0	9	0	1	0	2
156			min	-475.763	3	.372	15	-.015	1	0	3	0	3	0	3
157		3	max	571.205	2	1.407	4	.038	3	0	9	0	1	0	2
158			min	-475.815	3	.331	15	-.015	1	0	3	0	3	0	3
159		4	max	571.136	2	1.23	4	.038	3	0	9	0	1	0	2
160			min	-475.867	3	.289	15	-.015	1	0	3	0	3	0	3
161		5	max	571.066	2	1.054	4	.038	3	0	9	0	1	0	15
162			min	-475.919	3	.248	15	-.015	1	0	3	0	3	0	3
163		6	max	570.997	2	.877	4	.038	3	0	9	0	1	0	15
164			min	-475.971	3	.206	15	-.015	1	0	3	0	3	0	4
165		7	max	570.928	2	.7	4	.038	3	0	9	0	1	0	15
166			min	-476.023	3	.165	15	-.015	1	0	3	0	3	0	4
167		8	max	570.858	2	.523	4	.038	3	0	9	0	1	0	15
168			min	-476.075	3	.123	15	-.015	1	0	3	0	3	-.001	4
169		9	max	570.789	2	.346	4	.038	3	0	9	0	1	0	15
170			min	-476.127	3	.079	12	-.015	1	0	3	0	3	-.001	4
171		10	max	570.72	2	.208	2	.038	3	0	9	0	1	0	15
172			min	-476.179	3	.01	3	-.015	1	0	3	0	3	-.001	4
173		11	max	570.65	2	.07	2	.038	3	0	9	0	1	0	15
174			min	-476.231	3	-.094	3	-.015	1	0	3	0	3	-.001	4
175		12	max	570.581	2	-.043	15	.038	3	0	9	0	1	0	15
176			min	-476.283	3	-.197	3	-.015	1	0	3	0	3	-.001	4
177		13	max	570.512	2	-.085	15	.038	3	0	9	0	1	0	15
178			min	-476.335	3	-.361	4	-.015	1	0	3	0	3	-.001	4
179		14	max	570.442	2	-.126	15	.038	3	0	9	0	1	0	15
180			min	-476.387	3	-.538	4	-.015	1	0	3	0	3	-.001	4
181		15	max	570.373	2	-.168	15	.038	3	0	9	0	1	0	15
182			min	-476.439	3	-.715	4	-.015	1	0	3	0	3	0	4
183		16	max	570.304	2	-.21	15	.038	3	0	9	0	1	0	15
184			min	-476.491	3	-.892	4	-.015	1	0	3	0	3	0	4
185		17	max	570.235	2	-.251	15	.038	3	0	9	0	9	0	15
186			min	-476.543	3	-1.068	4	-.015	1	0	3	0	3	0	4
187		18	max	570.165	2	-.293	15	.038	3	0	9	0	9	0	15
188			min	-476.595	3	-1.245	4	-.015	1	0	3	0	3	0	4
189		19	max	570.096	2	-.334	15	.038	3	0	9	0	9	0	1
190			min	-476.647	3	-1.422	4	-.015	1	0	3	0	3	0	1
191	M8	1	max	712.43	1	0	1	.12	9	0	1	0	2	0	1
192			min	-39.034	3	0	1	-.686	3	0	1	0	3	0	1
193		2	max	712.495	1	0	1	.12	9	0	1	0	9	0	1
194			min	-38.985	3	0	1	-.686	3	0	1	0	3	0	1
195		3	max	712.56	1	0	1	.12	9	0	1	0	9	0	1
196			min	-38.937	3	0	1	-.686	3	0	1	0	3	0	1
197		4	max	712.625	1	0	1	.12	9	0	1	0	9	0	1
198			min	-38.888	3	0	1	-.686	3	0	1	0	3	0	1
199		5	max	712.689	1	0	1	.12	9	0	1	0	9	0	1
200			min	-38.84	3	0	1	-.686	3	0	1	0	3	0	1
201		6	max	712.754	1	0	1	.12	9	0	1	0	9	0	1
202			min	-38.791	3	0	1	-.686	3	0	1	0	3	0	1
203		7	max	712.819	1	0	1	.12	9	0	1	0	9	0	1
204			min	-38.743	3	0	1	-.686	3	0	1	0	3	0	1
205		8	max	712.883	1	0	1	.12	9	0	1	0	9	0	1
206			min	-38.694	3	0	1	-.686	3	0	1	0	3	0	1
207		9	max	712.948	1	0	1	.12	9	0	1	0	9	0	1
208			min	-38.646	3	0	1	-.686	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	713.013	1	0	1	.12	9	0	1	0	9	0	1
210		min	-38.597	3	0	1	-.686	3	0	1	0	3	0	1
211	11	max	713.077	1	0	1	.12	9	0	1	0	9	0	1
212		min	-38.549	3	0	1	-.686	3	0	1	0	3	0	1
213	12	max	713.142	1	0	1	.12	9	0	1	0	9	0	1
214		min	-38.5	3	0	1	-.686	3	0	1	0	3	0	1
215	13	max	713.207	1	0	1	.12	9	0	1	0	9	0	1
216		min	-38.452	3	0	1	-.686	3	0	1	0	3	0	1
217	14	max	713.272	1	0	1	.12	9	0	1	0	9	0	1
218		min	-38.403	3	0	1	-.686	3	0	1	0	3	0	1
219	15	max	713.336	1	0	1	.12	9	0	1	0	9	0	1
220		min	-38.355	3	0	1	-.686	3	0	1	0	3	0	1
221	16	max	713.401	1	0	1	.12	9	0	1	0	9	0	1
222		min	-38.306	3	0	1	-.686	3	0	1	0	3	0	1
223	17	max	713.466	1	0	1	.12	9	0	1	0	9	0	1
224		min	-38.257	3	0	1	-.686	3	0	1	0	3	0	1
225	18	max	713.53	1	0	1	.12	9	0	1	0	9	0	1
226		min	-38.209	3	0	1	-.686	3	0	1	-.001	3	0	1
227	19	max	713.595	1	0	1	.12	9	0	1	0	9	0	1
228		min	-38.16	3	0	1	-.686	3	0	1	-.001	3	0	1
229	M10	1	max	195.535	2	.656	.002	10	0	1	0	1	0	1
230		min	-273.066	3	.154	15	-.102	1	0	3	0	3	0	1
231	2	max	195.661	2	.605	4	.002	10	0	1	0	1	0	15
232		min	-272.971	3	.142	15	-.102	1	0	3	0	3	0	4
233	3	max	195.787	2	.554	4	.002	10	0	1	0	1	0	15
234		min	-272.877	3	.13	15	-.102	1	0	3	0	3	0	4
235	4	max	195.913	2	.502	4	.002	10	0	1	0	1	0	15
236		min	-272.782	3	.118	15	-.102	1	0	3	0	3	0	4
237	5	max	196.039	2	.451	4	.002	10	0	1	0	1	0	15
238		min	-272.688	3	.106	15	-.102	1	0	3	0	3	0	4
239	6	max	196.165	2	.4	4	.002	10	0	1	0	1	0	15
240		min	-272.594	3	.094	15	-.102	1	0	3	0	3	0	4
241	7	max	196.291	2	.349	4	.002	10	0	1	0	9	0	15
242		min	-272.499	3	.082	15	-.102	1	0	3	0	3	0	4
243	8	max	196.417	2	.298	4	.002	10	0	1	0	10	0	15
244		min	-272.405	3	.07	15	-.102	1	0	3	0	3	0	4
245	9	max	196.542	2	.247	4	.002	10	0	1	0	10	0	15
246		min	-272.31	3	.058	15	-.102	1	0	3	0	3	0	4
247	10	max	196.668	2	.196	4	.002	10	0	1	0	10	0	15
248		min	-272.216	3	.046	15	-.102	1	0	3	0	3	0	4
249	11	max	196.794	2	.144	4	.002	10	0	1	0	10	0	15
250		min	-272.122	3	.034	15	-.102	1	0	3	0	3	0	4
251	12	max	196.92	2	.103	2	.002	10	0	1	0	10	0	15
252		min	-272.027	3	.02	12	-.102	1	0	3	0	3	0	4
253	13	max	197.046	2	.063	2	.002	10	0	1	0	10	0	15
254		min	-271.933	3	-.001	3	-.102	1	0	3	0	3	0	4
255	14	max	197.172	2	.023	2	.002	10	0	1	0	10	0	15
256		min	-271.838	3	-.031	3	-.102	1	0	3	0	3	0	4
257	15	max	197.298	2	-.014	15	.002	10	0	1	0	10	0	15
258		min	-271.744	3	-.061	3	-.102	1	0	3	0	3	0	4
259	16	max	197.423	2	-.026	15	.002	10	0	1	0	10	0	15
260		min	-271.65	3	-.111	4	-.102	1	0	3	0	3	0	4
261	17	max	197.549	2	-.038	15	.002	10	0	1	0	10	0	15
262		min	-271.555	3	-.162	4	-.102	1	0	3	0	3	0	4
263	18	max	197.675	2	-.05	15	.002	10	0	1	0	10	0	15
264		min	-271.461	3	-.214	4	-.102	1	0	3	0	3	0	4
265	19	max	197.801	2	-.062	15	.002	10	0	1	0	10	0	15



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-271.366	3	-.265	4	-.102	1	0	3	0	3	0	4
267	M11	1	max	183.198	2	1.759	4	.125	1	0	3	0	3	0	4
268			min	-174.315	3	.414	15	-.056	3	0	10	0	1	0	15
269		2	max	183.129	2	1.582	4	.125	1	0	3	0	3	0	2
270			min	-174.367	3	.372	15	-.056	3	0	10	0	1	0	12
271		3	max	183.059	2	1.405	4	.125	1	0	3	0	3	0	2
272			min	-174.419	3	.33	15	-.056	3	0	10	0	1	0	3
273		4	max	182.99	2	1.228	4	.125	1	0	3	0	3	0	15
274			min	-174.471	3	.289	15	-.056	3	0	10	0	1	0	4
275		5	max	182.921	2	1.052	4	.125	1	0	3	0	3	0	15
276			min	-174.523	3	.247	15	-.056	3	0	10	0	1	0	4
277		6	max	182.852	2	.875	4	.125	1	0	3	0	3	0	15
278			min	-174.575	3	.206	15	-.056	3	0	10	0	1	0	4
279		7	max	182.782	2	.698	4	.125	1	0	3	0	3	0	15
280			min	-174.627	3	.164	15	-.056	3	0	10	0	1	0	4
281		8	max	182.713	2	.521	4	.125	1	0	3	0	3	0	15
282			min	-174.679	3	.123	15	-.056	3	0	10	0	1	-.001	4
283		9	max	182.644	2	.344	4	.125	1	0	3	0	3	0	15
284			min	-174.731	3	.081	15	-.056	3	0	10	0	1	-.001	4
285		10	max	182.574	2	.167	4	.125	1	0	3	0	3	0	15
286			min	-174.783	3	.039	15	-.056	3	0	10	0	1	-.001	4
287		11	max	182.505	2	.018	2	.125	1	0	3	0	3	0	15
288			min	-174.835	3	-.041	3	-.056	3	0	10	0	1	-.001	4
289		12	max	182.436	2	-.044	15	.125	1	0	3	0	3	0	15
290			min	-174.887	3	-.186	4	-.056	3	0	10	0	1	-.001	4
291		13	max	182.366	2	-.085	15	.125	1	0	3	0	3	0	15
292			min	-174.938	3	-.363	4	-.056	3	0	10	0	1	-.001	4
293		14	max	182.297	2	-.127	15	.125	1	0	3	0	3	0	15
294			min	-174.99	3	-.54	4	-.056	3	0	10	0	1	-.001	4
295		15	max	182.228	2	-.168	15	.125	1	0	3	0	3	0	15
296			min	-175.042	3	-.717	4	-.056	3	0	10	0	1	0	4
297		16	max	182.158	2	-.21	15	.125	1	0	3	0	3	0	15
298			min	-175.094	3	-.894	4	-.056	3	0	10	0	10	0	4
299		17	max	182.089	2	-.251	15	.125	1	0	3	0	3	0	15
300			min	-175.146	3	-1.07	4	-.056	3	0	10	0	10	0	4
301		18	max	182.02	2	-.293	15	.125	1	0	3	0	3	0	15
302			min	-175.198	3	-1.247	4	-.056	3	0	10	0	10	0	4
303		19	max	181.95	2	-.335	15	.125	1	0	3	0	3	0	1
304			min	-175.25	3	-1.424	4	-.056	3	0	10	0	10	0	1
305	M12	1	max	255.193	1	0	1	.604	1	0	1	0	2	0	1
306			min	3.265	12	0	1	-.037	10	0	1	0	3	0	1
307		2	max	255.257	1	0	1	.604	1	0	1	0	1	0	1
308			min	3.297	12	0	1	-.037	10	0	1	0	15	0	1
309		3	max	255.322	1	0	1	.604	1	0	1	0	1	0	1
310			min	3.33	12	0	1	-.037	10	0	1	0	10	0	1
311		4	max	255.387	1	0	1	.604	1	0	1	0	1	0	1
312			min	3.362	12	0	1	-.037	10	0	1	0	10	0	1
313		5	max	255.452	1	0	1	.604	1	0	1	0	1	0	1
314			min	3.394	12	0	1	-.037	10	0	1	0	10	0	1
315		6	max	255.516	1	0	1	.604	1	0	1	0	1	0	1
316			min	3.427	12	0	1	-.037	10	0	1	0	10	0	1
317		7	max	255.581	1	0	1	.604	1	0	1	0	1	0	1
318			min	3.459	12	0	1	-.037	10	0	1	0	10	0	1
319		8	max	255.646	1	0	1	.604	1	0	1	0	1	0	1
320			min	3.492	12	0	1	-.037	10	0	1	0	10	0	1
321		9	max	255.71	1	0	1	.604	1	0	1	0	1	0	1
322			min	3.524	12	0	1	-.037	10	0	1	0	10	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-70.532	1	-158.149	3	-15.74	1	0	2	-.03	1	0	3
381	M5	1	max	177.447	1	1046.757	3	0	11	0	.012	3	0	3
382		min	-4.325	3	-666.658	2	-79.58	3	0	3	0	11	0	2
383		2	max	177.586	1	1046.575	3	0	11	0	0	9	.144	2
384		min	-4.22	3	-666.9	2	-79.58	3	0	3	-.005	3	-.226	3
385		3	max	237.124	3	4.737	9	8.536	3	0	0	9	.286	2
386		min	-48.563	2	-80.008	2	-.138	9	0	9	-.022	3	-.448	3
387		4	max	237.229	3	4.536	9	8.536	3	0	0	9	.304	2
388		min	-48.424	2	-80.25	2	-.138	9	0	9	-.02	3	-.438	3
389		5	max	237.334	3	4.334	9	8.536	3	0	0	9	.321	2
390		min	-48.284	2	-80.492	2	-.138	9	0	9	-.018	3	-.428	3
391		6	max	237.438	3	4.132	9	8.536	3	0	0	9	.339	2
392		min	-48.144	2	-80.734	2	-.138	9	0	9	-.016	3	-.417	3
393		7	max	237.543	3	3.931	9	8.536	3	0	0	9	.356	2
394		min	-48.005	2	-80.976	2	-.138	9	0	9	-.014	3	-.407	3
395		8	max	237.648	3	3.729	9	8.536	3	0	0	9	.374	2
396		min	-47.865	2	-81.217	2	-.138	9	0	9	-.013	3	-.396	3
397		9	max	237.753	3	3.528	9	8.536	3	0	0	9	.391	2
398		min	-47.726	2	-81.459	2	-.138	9	0	9	-.011	3	-.385	3
399		10	max	237.857	3	3.326	9	8.536	3	0	0	1	.409	2
400		min	-47.586	2	-81.701	2	-.138	9	0	9	-.009	3	-.375	3
401		11	max	237.962	3	3.125	9	8.536	3	0	0	2	.427	2
402		min	-47.446	2	-81.943	2	-.138	9	0	9	-.007	3	-.364	3
403		12	max	238.067	3	2.923	9	8.536	3	0	0	2	.445	2
404		min	-47.307	2	-82.185	2	-.138	9	0	9	-.005	3	-.353	3
405		13	max	238.171	3	2.722	9	8.536	3	0	0	2	.463	2
406		min	-47.167	2	-82.427	2	-.138	9	0	9	-.003	3	-.343	3
407		14	max	238.276	3	2.52	9	8.536	3	0	0	2	.48	2
408		min	-47.027	2	-82.668	2	-.138	9	0	9	-.001	3	-.332	3
409		15	max	238.381	3	2.319	9	8.536	3	0	0	3	.498	2
410		min	-46.888	2	-82.91	2	-.138	9	0	9	0	9	-.321	3
411		16	max	266.777	2	395.749	2	8.512	3	0	.002	3	.512	2
412		min	-22.603	3	-449.066	3	-.14	9	0	2	0	9	-.306	3
413		17	max	266.917	2	395.507	2	8.512	3	0	.004	3	.426	2
414		min	-22.498	3	-449.247	3	-.14	9	0	2	0	9	-.209	3
415		18	max	-2.249	12	990.139	2	7.828	3	0	.005	3	.214	2
416		min	-177.598	1	-483.416	3	-.025	9	0	9	0	9	-.104	3
417		19	max	-2.179	12	989.897	2	7.828	3	0	.007	3	0	3
418		min	-177.459	1	-483.597	3	-.025	9	0	9	0	9	0	2
419	M9	1	max	70.509	1	330.545	3	84.573	3	0	.002	10	0	2
420		min	2.938	15	-213.689	2	-.814	10	0	2	-.03	1	0	3
421		2	max	70.649	1	330.363	3	84.573	3	0	.001	10	.047	2
422		min	2.98	15	-213.93	2	-.814	10	0	2	-.026	1	-.072	3
423		3	max	87.301	3	4.023	9	15.032	1	0	.015	3	.092	2
424		min	-16.841	10	-24.201	2	-2.709	3	0	10	-.023	1	-.142	3
425		4	max	87.406	3	3.822	9	15.032	1	0	.015	3	.098	2
426		min	-16.725	10	-24.443	2	-2.709	3	0	10	-.02	1	-.139	3
427		5	max	87.51	3	3.62	9	15.032	1	0	.014	3	.103	2
428		min	-16.608	10	-24.684	2	-2.709	3	0	10	-.016	1	-.137	3
429		6	max	87.615	3	3.419	9	15.032	1	0	.014	3	.108	2
430		min	-16.492	10	-24.926	2	-2.709	3	0	10	-.013	1	-.134	3
431		7	max	87.72	3	3.217	9	15.032	1	0	.013	3	.114	2
432		min	-16.376	10	-25.168	2	-2.709	3	0	10	-.01	1	-.131	3
433		8	max	87.824	3	3.016	9	15.032	1	0	.012	3	.119	2
434		min	-16.259	10	-25.41	2	-2.709	3	0	10	-.007	1	-.129	3
435		9	max	87.929	3	2.814	9	15.032	1	0	.012	3	.125	2
436		min	-16.143	10	-25.652	2	-2.709	3	0	10	-.003	1	-.126	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	88.034	3	2.612	9	15.032	1	0	1	.011	3	.13	2
438		min	-16.027	10	-25.894	2	-2.709	3	0	10	0	1	-.123	3
439	11	max	88.139	3	2.411	9	15.032	1	0	1	.011	3	.136	2
440		min	-15.91	10	-26.135	2	-2.709	3	0	10	0	10	-.12	3
441	12	max	88.243	3	2.209	9	15.032	1	0	1	.01	3	.142	2
442		min	-15.794	10	-26.377	2	-2.709	3	0	10	0	10	-.117	3
443	13	max	88.348	3	2.008	9	15.032	1	0	1	.01	1	.147	2
444		min	-15.678	10	-26.619	2	-2.709	3	0	10	0	10	-.114	3
445	14	max	88.453	3	1.806	9	15.032	1	0	1	.013	1	.153	2
446		min	-15.561	10	-26.861	2	-2.709	3	0	10	0	10	-.111	3
447	15	max	88.557	3	1.605	9	15.032	1	0	1	.016	1	.159	2
448		min	-15.445	10	-27.103	2	-2.709	3	0	10	0	10	-.108	3
449	16	max	86.372	2	124.45	2	15.143	1	0	10	.02	1	.164	2
450		min	-6.84	3	-161.26	3	-2.76	3	0	3	-.001	10	-.104	3
451	17	max	86.511	2	124.208	2	15.143	1	0	10	.023	1	.137	2
452		min	-6.735	3	-161.442	3	-2.76	3	0	3	-.001	10	-.069	3
453	18	max	-2.979	15	316.063	2	15.756	1	0	2	.026	1	.069	2
454		min	-70.638	1	-157.957	3	-2.306	3	0	3	-.001	10	-.035	3
455	19	max	-2.937	15	315.821	2	15.756	1	0	2	.03	1	0	2
456		min	-70.499	1	-158.138	3	-2.306	3	0	3	-.002	10	0	3
457	M13	1	max	84.566	3	213.614	2	-2.938	15	0	.03	1	0	2
458		min	-.814	10	-330.6	3	-70.505	1	0	3	-.002	10	0	3
459	2	max	84.566	3	152.886	2	-2.226	15	0	2	.016	3	.118	3
460		min	-.814	10	-235.716	3	-52.811	1	0	3	-.004	2	-.076	2
461	3	max	84.566	3	92.159	2	-1.023	10	0	2	.012	3	.196	3
462		min	-.814	10	-140.833	3	-35.118	1	0	3	-.014	1	-.127	2
463	4	max	84.566	3	31.431	2	.945	10	0	2	.009	3	.235	3
464		min	-.814	10	-45.949	3	-17.424	1	0	3	-.025	1	-.153	2
465	5	max	84.566	3	48.934	3	4.152	2	0	2	.006	3	.235	3
466		min	-.814	10	-29.297	2	-6.903	3	0	3	-.029	1	-.154	2
467	6	max	84.566	3	143.818	3	17.964	1	0	2	.003	3	.195	3
468		min	-.814	10	-90.025	2	-5.867	3	0	3	-.025	1	-.129	2
469	7	max	84.566	3	238.701	3	35.658	1	0	2	0	10	.115	3
470		min	-.814	10	-150.753	2	-4.83	3	0	3	-.014	1	-.079	2
471	8	max	84.566	3	333.585	3	53.352	1	0	2	.007	2	0	9
472		min	-.814	10	-211.48	2	-3.794	3	0	3	-.001	3	-.004	3
473	9	max	84.566	3	428.468	3	71.046	1	0	2	.031	1	.098	2
474		min	-.814	10	-272.208	2	-2.757	3	0	3	-.003	3	-.163	3
475	10	max	84.566	3	-6.797	15	88.74	1	0	2	.064	1	.224	2
476		min	-.814	10	-523.352	3	1.44	12	0	3	-.015	3	-.361	3
477	11	max	15.172	1	272.208	2	3.672	3	0	3	.031	1	.098	2
478		min	-.814	10	-428.468	3	-71.021	1	0	2	-.014	3	-.163	3
479	12	max	15.172	1	211.48	2	4.709	3	0	3	.007	2	0	9
480		min	-.814	10	-333.585	3	-53.327	1	0	2	-.012	3	-.004	3
481	13	max	15.172	1	150.753	2	5.745	3	0	3	0	10	.115	3
482		min	-.814	10	-238.701	3	-35.633	1	0	2	-.014	1	-.079	2
483	14	max	15.172	1	90.025	2	6.782	3	0	3	-.001	15	.195	3
484		min	-.814	10	-143.818	3	-17.94	1	0	2	-.025	1	-.129	2
485	15	max	15.172	1	29.297	2	7.818	3	0	3	-.001	15	.235	3
486		min	-.814	10	-48.934	3	-4.152	2	0	2	-.029	1	-.154	2
487	16	max	15.172	1	45.949	3	17.448	1	0	3	0	12	.235	3
488		min	-.814	10	-31.431	2	-.945	10	0	2	-.025	1	-.153	2
489	17	max	15.172	1	140.833	3	35.142	1	0	3	.003	3	.196	3
490		min	-.814	10	-92.159	2	1.023	10	0	2	-.014	1	-.127	2
491	18	max	15.172	1	235.716	3	52.836	1	0	3	.007	3	.118	3
492		min	-.814	10	-152.886	2	2.232	15	0	2	-.004	2	-.076	2
493	19	max	15.172	1	330.6	3	70.53	1	0	3	.03	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-814	10	-213.614	2	2.945	15	0	2	-.002	10	0	3
495	M16	1	max	2.31	3	315.916	2	-2.937	15	0	3	.03	1	0	2
496			min	-15.73	1	-158.166	3	-70.503	1	0	2	-.002	10	0	3
497		2	max	2.31	3	225.764	2	-2.225	15	0	3	.005	9	.057	3
498			min	-15.73	1	-113.724	3	-52.809	1	0	2	-.004	2	-.113	2
499		3	max	2.31	3	135.612	2	-1.036	10	0	3	0	3	.095	3
500			min	-15.73	1	-69.282	3	-35.115	1	0	2	-.014	1	-.188	2
501		4	max	2.31	3	45.46	2	.931	10	0	3	0	15	.114	3
502			min	-15.73	1	-24.84	3	-17.421	1	0	2	-.025	1	-.226	2
503		5	max	2.31	3	19.602	3	4.131	2	0	3	-.001	15	.115	3
504			min	-15.73	1	-44.691	2	-4.49	3	0	2	-.029	1	-.226	2
505		6	max	2.31	3	64.045	3	17.966	1	0	3	-.001	15	.098	3
506			min	-15.73	1	-134.843	2	-3.454	3	0	2	-.025	1	-.189	2
507		7	max	2.31	3	108.487	3	35.66	1	0	3	0	10	.062	3
508			min	-15.73	1	-224.995	2	-2.417	3	0	2	-.014	1	-.114	2
509		8	max	2.31	3	152.929	3	53.354	1	0	3	.007	2	.008	3
510			min	-15.73	1	-315.147	2	-1.381	3	0	2	-.008	3	-.001	2
511		9	max	2.31	3	197.371	3	71.048	1	0	3	.031	1	.149	2
512			min	-15.73	1	-405.299	2	-.345	3	0	2	-.008	3	-.065	3
513		10	max	.854	10	-6.793	15	88.742	1	0	15	.064	1	.337	2
514			min	-15.73	1	-495.45	2	-2.127	3	0	2	-.008	3	-.157	3
515		11	max	.854	10	405.299	2	-.845	12	0	2	.031	1	.149	2
516			min	-15.714	1	-197.371	3	-71.015	1	0	3	-.002	3	-.065	3
517		12	max	.854	10	315.147	2	-.054	3	0	2	.007	2	.008	3
518			min	-15.714	1	-152.929	3	-53.321	1	0	3	-.002	3	-.001	2
519		13	max	.854	10	224.995	2	.982	3	0	2	0	10	.062	3
520			min	-15.714	1	-108.487	3	-35.627	1	0	3	-.014	1	-.114	2
521		14	max	.854	10	134.843	2	2.019	3	0	2	0	12	.098	3
522			min	-15.714	1	-64.045	3	-17.933	1	0	3	-.025	1	-.189	2
523		15	max	.854	10	44.691	2	3.055	3	0	2	0	12	.115	3
524			min	-15.714	1	-19.602	3	-4.131	2	0	3	-.029	1	-.226	2
525		16	max	.854	10	24.84	3	17.455	1	0	2	.001	3	.114	3
526			min	-15.714	1	-45.46	2	-.931	10	0	3	-.025	1	-.226	2
527		17	max	.854	10	69.282	3	35.149	1	0	2	.003	3	.095	3
528			min	-15.714	1	-135.612	2	1.036	10	0	3	-.014	1	-.188	2
529		18	max	.854	10	113.724	3	52.843	1	0	2	.005	3	.057	3
530			min	-15.714	1	-225.764	2	2.231	15	0	3	-.004	2	-.113	2
531		19	max	.854	10	158.166	3	70.537	1	0	2	.03	1	0	2
532			min	-15.714	1	-315.916	2	2.944	15	0	3	-.002	10	0	3
533	M15	1	max	0	1	.785	3	.145	3	0	1	0	1	0	1
534			min	-117.062	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.698	3	.145	3	0	1	0	1	0	1
536			min	-117.133	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.611	3	.145	3	0	1	0	1	0	1
538			min	-117.203	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.524	3	.145	3	0	1	0	1	0	1
540			min	-117.274	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.436	3	.145	3	0	1	0	1	0	1
542			min	-117.344	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.349	3	.145	3	0	1	0	1	0	1
544			min	-117.415	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.262	3	.145	3	0	1	0	3	0	1
546			min	-117.485	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.175	3	.145	3	0	1	0	3	0	1
548			min	-117.556	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.087	3	.145	3	0	1	0	3	0	1
550			min	-117.626	3	0	1	0	1	0	3	0	1	0	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.145	3	0	1	0	3	0	1
552		min	-117.697	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.145	3	0	1	0	3	0	1
554		min	-117.767	3	-.087	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.145	3	0	1	0	3	0	1
556		min	-117.838	3	-.175	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.145	3	0	1	0	3	0	1
558		min	-117.908	3	-.262	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.145	3	0	1	0	3	0	1
560		min	-117.979	3	-.349	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.145	3	0	1	0	3	0	1
562		min	-118.049	3	-.436	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.145	3	0	1	0	3	0	1
564		min	-118.12	3	-.524	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.145	3	0	1	0	3	0	1
566		min	-118.19	3	-.611	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.145	3	0	1	0	3	0	1
568		min	-118.261	3	-.698	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.145	3	0	1	0	3	0	1
570		min	-118.331	3	-.785	3	0	1	0	3	0	1	0	1
571	M16A 1	max	0	2	1.344	4	.04	1	0	3	0	3	0	1
572		min	-116.646	3	0	2	-.06	3	0	1	0	1	0	1
573	2	max	0	2	1.195	4	.04	1	0	3	0	3	0	2
574		min	-116.576	3	0	2	-.06	3	0	1	0	1	0	4
575	3	max	0	2	1.045	4	.04	1	0	3	0	3	0	2
576		min	-116.505	3	0	2	-.06	3	0	1	0	1	0	4
577	4	max	0	2	.896	4	.04	1	0	3	0	3	0	2
578		min	-116.435	3	0	2	-.06	3	0	1	0	1	0	4
579	5	max	0	2	.747	4	.04	1	0	3	0	3	0	2
580		min	-116.364	3	0	2	-.06	3	0	1	0	1	-.001	4
581	6	max	0	2	.597	4	.04	1	0	3	0	3	0	2
582		min	-116.294	3	0	2	-.06	3	0	1	0	1	-.001	4
583	7	max	0	2	.448	4	.04	1	0	3	0	3	0	2
584		min	-116.223	3	0	2	-.06	3	0	1	0	1	-.001	4
585	8	max	0	2	.299	4	.04	1	0	3	0	3	0	2
586		min	-116.153	3	0	2	-.06	3	0	1	0	1	-.002	4
587	9	max	0	2	.149	4	.04	1	0	3	0	3	0	2
588		min	-116.082	3	0	2	-.06	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.04	1	0	3	0	3	0	2
590		min	-116.012	3	0	1	-.06	3	0	1	0	1	-.002	4
591	11	max	.004	13	0	2	.04	1	0	3	0	3	0	2
592		min	-115.941	3	-.149	4	-.06	3	0	1	0	1	-.002	4
593	12	max	.101	13	0	2	.04	1	0	3	0	3	0	2
594		min	-115.871	3	-.299	4	-.06	3	0	1	0	1	-.002	4
595	13	max	.198	13	0	2	.04	1	0	3	0	1	0	2
596		min	-115.8	3	-.448	4	-.06	3	0	1	0	4	-.001	4
597	14	max	.295	13	0	2	.04	1	0	3	0	1	0	2
598		min	-115.73	3	-.597	4	-.06	3	0	1	0	3	-.001	4
599	15	max	.395	4	0	2	.04	1	0	3	0	1	0	2
600		min	-115.66	3	-.747	4	-.06	3	0	1	0	3	-.001	4
601	16	max	.515	4	0	2	.04	1	0	3	0	1	0	2
602		min	-115.589	3	-.896	4	-.06	3	0	1	0	3	0	4
603	17	max	.636	4	0	2	.04	1	0	3	0	1	0	2
604		min	-115.519	3	-1.045	4	-.06	3	0	1	0	3	0	4
605	18	max	.757	4	0	2	.04	1	0	3	0	1	0	2
606		min	-115.448	3	-1.195	4	-.06	3	0	1	0	3	0	4
607	19	max	.877	4	0	2	.04	1	0	3	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-115.378	3	-1.344	4	-0.06	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.009	2	.002	9	1.365e-5	10	NC	3	NC	1	
2			min	-.003	3	-.009	3	-.002	3	-2.438e-4	1	4314.383	2	NC	1	
3			2	max	.002	2	.008	2	.002	9	1.3e-5	10	NC	3	NC	1
4				min	-.003	3	-.009	3	-.002	3	-2.325e-4	1	4712.473	2	NC	1
5			3	max	.002	2	.008	2	.002	9	1.235e-5	10	NC	1	NC	1
6				min	-.003	3	-.008	3	-.002	3	-2.211e-4	1	5186.689	2	NC	1
7			4	max	.002	2	.007	2	.002	1	1.171e-5	10	NC	1	NC	1
8				min	-.003	3	-.008	3	-.002	3	-2.098e-4	1	5755.592	2	NC	1
9			5	max	.001	2	.006	2	.001	1	1.106e-5	10	NC	1	NC	1
10				min	-.003	3	-.008	3	-.001	3	-1.984e-4	1	6443.985	2	NC	1
11		6	max	.001	2	.005	2	.001	1	1.041e-5	10	NC	1	NC	1	
12			min	-.002	3	-.007	3	-.001	3	-1.871e-4	1	7285.556	2	NC	1	
13		7	max	.001	2	.005	2	.001	1	9.766e-6	10	NC	1	NC	1	
14			min	-.002	3	-.007	3	-.001	3	-1.758e-4	1	8326.95	2	NC	1	
15		8	max	.001	2	.004	2	.001	1	9.119e-6	10	NC	1	NC	1	
16			min	-.002	3	-.006	3	0	3	-1.644e-4	1	9634.213	2	NC	1	
17		9	max	.001	2	.003	2	0	1	8.472e-6	10	NC	1	NC	1	
18			min	-.002	3	-.006	3	0	3	-1.531e-4	1	NC	1	NC	1	
19		10	max	0	2	.003	2	0	1	7.825e-6	10	NC	1	NC	1	
20			min	-.002	3	-.006	3	0	3	-1.417e-4	1	NC	1	NC	1	
21		11	max	0	2	.002	2	0	1	7.178e-6	10	NC	1	NC	1	
22			min	-.002	3	-.005	3	0	3	-1.304e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	0	1	6.531e-6	10	NC	1	NC	1	
24			min	-.001	3	-.004	3	0	3	-1.191e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	1	5.884e-6	10	NC	1	NC	1	
26			min	-.001	3	-.004	3	0	3	-1.077e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	1	5.237e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-9.638e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	1	4.59e-6	10	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-8.504e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	1	3.943e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-7.37e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	1	3.296e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-6.236e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	1	2.649e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-5.102e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.002e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.968e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.88e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-9.506e-7	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	2.603e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	9	-1.324e-6	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	3.326e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	9	-1.696e-6	10	NC	1	NC	1
45			4	max	0	3	0	2	0	3	4.049e-5	1	NC	1	NC	1
46				min	0	2	-.003	3	0	9	-2.069e-6	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	4.771e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	9	-2.442e-6	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	5.494e-5	1	NC	1	NC	1
50				min	0	2	-.004	3	0	9	-2.815e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	6.217e-5	1	NC	1	NC	1	



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	-3.188e-6	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	6.94e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-3.561e-6	10	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	7.662e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-3.934e-6	10	NC	1	NC	1
57		10	max	0	3	.002	2	0	1	8.385e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-4.307e-6	10	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	9.108e-5	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-4.68e-6	10	NC	1	NC	1
61		12	max	.001	3	.003	2	0	1	9.831e-5	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	10	-5.053e-6	10	NC	1	NC	1
63		13	max	.001	3	.003	2	0	1	1.055e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-5.426e-6	10	NC	1	NC	1
65		14	max	.001	3	.004	2	.001	1	1.128e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-5.799e-6	10	NC	1	NC	1
67		15	max	.002	3	.005	2	.001	1	1.2e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-6.172e-6	10	9487.261	2	NC	1
69		16	max	.002	3	.006	2	.002	1	1.272e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-6.545e-6	10	7995.276	2	NC	1
71		17	max	.002	3	.007	2	.002	1	1.344e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-6.918e-6	10	6850.444	2	NC	1
73		18	max	.002	3	.008	2	.002	1	1.417e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-7.291e-6	10	5960.796	2	NC	1
75		19	max	.002	3	.009	2	.002	1	1.489e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-7.663e-6	10	5262.552	2	NC	1
77	M4	1	max	.001	1	.01	2	0	10	9.268e-6	10	NC	1	NC	1
78			min	0	12	-.009	3	-.002	1	-1.788e-4	1	NC	1	NC	1
79		2	max	.001	1	.01	2	0	10	9.268e-6	10	NC	1	NC	1
80			min	0	12	-.009	3	-.002	1	-1.788e-4	1	NC	1	NC	1
81		3	max	.001	1	.009	2	0	10	9.268e-6	10	NC	1	NC	1
82			min	0	12	-.008	3	-.001	1	-1.788e-4	1	NC	1	NC	1
83		4	max	.001	1	.009	2	0	10	9.268e-6	10	NC	1	NC	1
84			min	0	12	-.008	3	-.001	1	-1.788e-4	1	NC	1	NC	1
85		5	max	0	1	.008	2	0	10	9.268e-6	10	NC	1	NC	1
86			min	0	12	-.007	3	-.001	1	-1.788e-4	1	NC	1	NC	1
87		6	max	0	1	.008	2	0	10	9.268e-6	10	NC	1	NC	1
88			min	0	12	-.007	3	0	1	-1.788e-4	1	NC	1	NC	1
89		7	max	0	1	.007	2	0	10	9.268e-6	10	NC	1	NC	1
90			min	0	12	-.006	3	0	1	-1.788e-4	1	NC	1	NC	1
91		8	max	0	1	.006	2	0	10	9.268e-6	10	NC	1	NC	1
92			min	0	12	-.006	3	0	1	-1.788e-4	1	NC	1	NC	1
93		9	max	0	1	.006	2	0	10	9.268e-6	10	NC	1	NC	1
94			min	0	12	-.005	3	0	1	-1.788e-4	1	NC	1	NC	1
95		10	max	0	1	.005	2	0	10	9.268e-6	10	NC	1	NC	1
96			min	0	12	-.005	3	0	1	-1.788e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	10	9.268e-6	10	NC	1	NC	1
98			min	0	12	-.004	3	0	1	-1.788e-4	1	NC	1	NC	1
99		12	max	0	1	.004	2	0	10	9.268e-6	10	NC	1	NC	1
100			min	0	12	-.004	3	0	1	-1.788e-4	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	10	9.268e-6	10	NC	1	NC	1
102			min	0	12	-.003	3	0	1	-1.788e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	10	9.268e-6	10	NC	1	NC	1
104			min	0	12	-.003	3	0	1	-1.788e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	10	9.268e-6	10	NC	1	NC	1
106			min	0	12	-.002	3	0	1	-1.788e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	10	9.268e-6	10	NC	1	NC	1
108			min	0	12	-.002	3	0	1	-1.788e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	9.268e-6	10	NC	1	NC	1
110			min	0	12	-.001	3	0	1	-1.788e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	9.268e-6	10	NC	1	NC	1
112			min	0	12	0	3	0	1	-1.788e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	9.268e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.788e-4	1	NC	1	NC	1
115	M6	1	max	.006	2	.029	2	0	9	4.806e-4	3	NC	3	NC	1
116			min	-.01	3	-.027	3	-.006	3	-8.672e-8	2	1378.227	2	6164.499	3
117		2	max	.006	2	.027	2	0	9	4.659e-4	3	NC	3	NC	1
118			min	-.01	3	-.025	3	-.006	3	-1.142e-6	1	1476.157	2	6538.924	3
119		3	max	.005	2	.025	2	0	9	4.512e-4	3	NC	3	NC	1
120			min	-.009	3	-.024	3	-.006	3	-2.243e-6	1	1588.564	2	6984.923	3
121		4	max	.005	2	.023	2	0	9	4.365e-4	3	NC	3	NC	1
122			min	-.008	3	-.023	3	-.005	3	-3.344e-6	1	1718.361	2	7517.671	3
123		5	max	.005	2	.021	2	0	9	4.218e-4	3	NC	3	NC	1
124			min	-.008	3	-.021	3	-.005	3	-4.446e-6	1	1869.3	2	8157.336	3
125		6	max	.004	2	.019	2	0	9	4.071e-4	3	NC	3	NC	1
126			min	-.007	3	-.02	3	-.004	3	-5.547e-6	1	2046.296	2	8931.109	3
127		7	max	.004	2	.017	2	0	9	3.925e-4	3	NC	3	NC	1
128			min	-.007	3	-.018	3	-.004	3	-6.648e-6	1	2255.912	2	9876.278	3
129		8	max	.004	2	.016	2	0	9	3.778e-4	3	NC	3	NC	1
130			min	-.006	3	-.017	3	-.004	3	-7.749e-6	1	2507.098	2	NC	1
131		9	max	.003	2	.014	2	0	9	3.631e-4	3	NC	3	NC	1
132			min	-.006	3	-.015	3	-.003	3	-8.85e-6	1	2812.393	2	NC	1
133		10	max	.003	2	.012	2	0	9	3.484e-4	3	NC	3	NC	1
134			min	-.005	3	-.014	3	-.003	3	-9.951e-6	1	3189.907	2	NC	1
135		11	max	.003	2	.011	2	0	9	3.337e-4	3	NC	3	NC	1
136			min	-.004	3	-.012	3	-.002	3	-1.105e-5	1	3666.802	2	NC	1
137		12	max	.002	2	.009	2	0	9	3.19e-4	3	NC	3	NC	1
138			min	-.004	3	-.011	3	-.002	3	-1.215e-5	1	4285.752	2	NC	1
139		13	max	.002	2	.008	2	0	9	3.043e-4	3	NC	3	NC	1
140			min	-.003	3	-.009	3	-.002	3	-1.325e-5	1	5117.861	2	NC	1
141		14	max	.002	2	.006	2	0	9	2.897e-4	3	NC	1	NC	1
142			min	-.003	3	-.008	3	-.001	3	-1.436e-5	1	6291.084	2	NC	1
143		15	max	.001	2	.005	2	0	9	2.75e-4	3	NC	1	NC	1
144			min	-.002	3	-.006	3	0	3	-1.546e-5	1	8061.27	2	NC	1
145		16	max	0	2	.004	2	0	9	2.603e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-1.656e-5	1	NC	1	NC	1
147		17	max	0	2	.002	2	0	9	2.456e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-1.766e-5	1	NC	1	NC	1
149		18	max	0	2	.001	2	0	1	2.309e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-1.876e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.162e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.986e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	9.377e-6	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.015e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	8.964e-6	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-7.65e-5	3	NC	1	NC	1
157		3	max	0	3	.002	2	0	3	8.55e-6	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-5.154e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	8.137e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.657e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	7.723e-6	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-1.609e-6	3	9601.845	2	NC	1
163		6	max	.002	3	.006	2	.002	3	2.336e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	0	2	7688.377	2	NC	1
165		7	max	.002	3	.007	2	.002	3	4.832e-5	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.011	3	0	9	0	5	6376.906	2	NC	1
167		8	max	.002	3	.009	2	.003	3	7.329e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	-1.283e-7	13	5413.612	2	NC	1
169		9	max	.002	3	.01	2	.003	3	9.825e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	9	-4.715e-7	9	4672.079	2	NC	1
171		10	max	.003	3	.011	2	.003	3	1.232e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	9	-1.417e-6	9	4082.222	2	NC	1
173		11	max	.003	3	.013	2	.003	3	1.482e-4	3	NC	3	NC	1
174			min	-.004	2	-.016	3	0	9	-2.362e-6	9	3601.9	2	NC	1
175		12	max	.003	3	.014	2	.003	3	1.731e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	9	-3.308e-6	9	3204.072	2	NC	1
177		13	max	.004	3	.016	2	.003	3	1.981e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	0	9	-4.254e-6	9	2870.429	2	NC	1
179		14	max	.004	3	.018	2	.003	3	2.231e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-5.199e-6	9	2588.003	2	NC	1
181		15	max	.004	3	.02	2	.003	3	2.48e-4	3	NC	3	NC	1
182			min	-.005	2	-.021	3	0	9	-6.145e-6	9	2347.258	2	NC	1
183		16	max	.005	3	.022	2	.003	3	2.73e-4	3	NC	3	NC	1
184			min	-.005	2	-.022	3	0	9	-7.09e-6	9	2140.97	2	NC	1
185		17	max	.005	3	.023	2	.003	3	2.98e-4	3	NC	3	NC	1
186			min	-.006	2	-.022	3	0	9	-8.036e-6	9	1963.531	2	NC	1
187		18	max	.005	3	.025	2	.003	3	3.229e-4	3	NC	3	NC	1
188			min	-.006	2	-.023	3	0	9	-8.981e-6	9	1810.507	2	NC	1
189		19	max	.005	3	.027	2	.003	3	3.479e-4	3	NC	3	NC	1
190			min	-.006	2	-.024	3	0	9	-9.927e-6	9	1678.348	2	NC	1
191	M8	1	max	.003	1	.033	2	0	9	-1.122e-7	10	NC	1	NC	1
192			min	0	3	-.027	3	-.002	3	-2.545e-4	3	NC	1	8919.046	3
193		2	max	.003	1	.031	2	0	9	-1.122e-7	10	NC	1	NC	1
194			min	0	3	-.025	3	-.002	3	-2.545e-4	3	NC	1	9724.604	3
195		3	max	.003	1	.029	2	0	9	-1.122e-7	10	NC	1	NC	1
196			min	0	3	-.024	3	-.002	3	-2.545e-4	3	NC	1	NC	1
197		4	max	.003	1	.027	2	0	9	-1.122e-7	10	NC	1	NC	1
198			min	0	3	-.022	3	-.002	3	-2.545e-4	3	NC	1	NC	1
199		5	max	.003	1	.025	2	0	9	-1.122e-7	10	NC	1	NC	1
200			min	0	3	-.021	3	-.001	3	-2.545e-4	3	NC	1	NC	1
201		6	max	.002	1	.024	2	0	9	-1.122e-7	10	NC	1	NC	1
202			min	0	3	-.019	3	-.001	3	-2.545e-4	3	NC	1	NC	1
203		7	max	.002	1	.022	2	0	9	-1.122e-7	10	NC	1	NC	1
204			min	0	3	-.018	3	-.001	3	-2.545e-4	3	NC	1	NC	1
205		8	max	.002	1	.02	2	0	9	-1.122e-7	10	NC	1	NC	1
206			min	0	3	-.016	3	0	3	-2.545e-4	3	NC	1	NC	1
207		9	max	.002	1	.018	2	0	9	-1.122e-7	10	NC	1	NC	1
208			min	0	3	-.015	3	0	3	-2.545e-4	3	NC	1	NC	1
209		10	max	.002	1	.016	2	0	9	-1.122e-7	10	NC	1	NC	1
210			min	0	3	-.013	3	0	3	-2.545e-4	3	NC	1	NC	1
211		11	max	.002	1	.014	2	0	9	-1.122e-7	10	NC	1	NC	1
212			min	0	3	-.012	3	0	3	-2.545e-4	3	NC	1	NC	1
213		12	max	.001	1	.013	2	0	9	-1.122e-7	10	NC	1	NC	1
214			min	0	3	-.01	3	0	3	-2.545e-4	3	NC	1	NC	1
215		13	max	.001	1	.011	2	0	9	-1.122e-7	10	NC	1	NC	1
216			min	0	3	-.009	3	0	3	-2.545e-4	3	NC	1	NC	1
217		14	max	0	1	.009	2	0	9	-1.122e-7	10	NC	1	NC	1
218			min	0	3	-.007	3	0	3	-2.545e-4	3	NC	1	NC	1
219		15	max	0	1	.007	2	0	9	-1.122e-7	10	NC	1	NC	1
220			min	0	3	-.006	3	0	3	-2.545e-4	3	NC	1	NC	1
221		16	max	0	1	.005	2	0	9	-1.122e-7	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-2.545e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-6.279e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	3.632e-6	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-7.06e-5	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	4.015e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-9.209e-5	3	NC	1	NC	1
285		10	max	0	3	.002	2	0	10	4.398e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.164e-4	3	NC	1	NC	1
287		11	max	.001	3	.002	2	0	10	4.781e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-1.406e-4	3	NC	1	NC	1
289		12	max	.001	3	.003	2	0	10	5.164e-6	10	NC	1	NC	1
290			min	-.001	2	-.007	3	-.003	3	-1.649e-4	3	NC	1	NC	1
291		13	max	.001	3	.003	2	0	10	5.547e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-1.891e-4	3	NC	1	NC	1
293		14	max	.001	3	.004	2	0	10	5.93e-6	10	NC	1	NC	1
294			min	-.001	2	-.008	3	-.003	3	-2.134e-4	3	NC	1	NC	1
295		15	max	.002	3	.005	2	0	10	6.313e-6	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.003	3	-2.377e-4	3	9500.451	2	NC	1
297		16	max	.002	3	.006	2	0	10	6.696e-6	10	NC	1	NC	1
298			min	-.002	2	-.008	3	-.003	3	-2.619e-4	3	8005.267	2	NC	1
299		17	max	.002	3	.007	2	0	10	7.079e-6	10	NC	1	NC	1
300			min	-.002	2	-.008	3	-.003	3	-2.862e-4	3	6858.235	2	NC	1
301		18	max	.002	3	.008	2	0	10	7.462e-6	10	NC	1	NC	1
302			min	-.002	2	-.008	3	-.003	3	-3.105e-4	3	5967.039	2	NC	1
303		19	max	.002	3	.009	2	0	10	7.845e-6	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.002	3	-3.347e-4	3	5267.687	2	NC	1
305	M12	1	max	.001	1	.01	2	.002	1	3.649e-4	3	NC	1	NC	2
306			min	0	12	-.009	3	0	10	-9.493e-6	10	NC	1	9974.246	1
307		2	max	.001	1	.01	2	.002	1	3.649e-4	3	NC	1	NC	1
308			min	0	12	-.009	3	0	10	-9.493e-6	10	NC	1	NC	1
309		3	max	.001	1	.009	2	.002	1	3.649e-4	3	NC	1	NC	1
310			min	0	12	-.008	3	0	10	-9.493e-6	10	NC	1	NC	1
311		4	max	.001	1	.009	2	.001	1	3.649e-4	3	NC	1	NC	1
312			min	0	12	-.008	3	0	10	-9.493e-6	10	NC	1	NC	1
313		5	max	0	1	.008	2	.001	1	3.649e-4	3	NC	1	NC	1
314			min	0	12	-.007	3	0	10	-9.493e-6	10	NC	1	NC	1
315		6	max	0	1	.008	2	.001	1	3.649e-4	3	NC	1	NC	1
316			min	0	12	-.007	3	0	10	-9.493e-6	10	NC	1	NC	1
317		7	max	0	1	.007	2	.001	1	3.649e-4	3	NC	1	NC	1
318			min	0	12	-.006	3	0	10	-9.493e-6	10	NC	1	NC	1
319		8	max	0	1	.006	2	0	1	3.649e-4	3	NC	1	NC	1
320			min	0	12	-.006	3	0	10	-9.493e-6	10	NC	1	NC	1
321		9	max	0	1	.006	2	0	1	3.649e-4	3	NC	1	NC	1
322			min	0	12	-.005	3	0	10	-9.493e-6	10	NC	1	NC	1
323		10	max	0	1	.005	2	0	1	3.649e-4	3	NC	1	NC	1
324			min	0	12	-.005	3	0	10	-9.493e-6	10	NC	1	NC	1
325		11	max	0	1	.005	2	0	1	3.649e-4	3	NC	1	NC	1
326			min	0	12	-.004	3	0	10	-9.493e-6	10	NC	1	NC	1
327		12	max	0	1	.004	2	0	1	3.649e-4	3	NC	1	NC	1
328			min	0	12	-.004	3	0	10	-9.493e-6	10	NC	1	NC	1
329		13	max	0	1	.003	2	0	1	3.649e-4	3	NC	1	NC	1
330			min	0	12	-.003	3	0	10	-9.493e-6	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	3.649e-4	3	NC	1	NC	1
332			min	0	12	-.003	3	0	10	-9.493e-6	10	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.649e-4	3	NC	1	NC	1
334			min	0	12	-.002	3	0	10	-9.493e-6	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	3.649e-4	3	NC	1	NC	1
336			min	0	12	-.002	3	0	10	-9.493e-6	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.649e-4	3	NC	1	NC	1
338			min	0	12	-.001	3	0	10	-9.493e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.649e-4	3	NC	1	NC	1
340			min	0	12	0	3	0	10	-9.493e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.649e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-9.493e-6	10	NC	1	NC	1
343	M1	1	max	.008	3	.024	3	.004	3	5.216e-3	2	NC	1	NC	1
344			min	-.009	2	-.02	2	0	9	-7.616e-3	3	NC	1	NC	1
345		2	max	.008	3	.014	3	.003	3	2.575e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.002	9	-3.748e-3	3	4868.141	3	NC	1
347		3	max	.008	3	.005	3	.002	3	4.755e-5	3	NC	4	NC	1
348			min	-.009	2	-.004	2	-.002	9	-1.13e-4	1	2521.594	3	NC	1
349		4	max	.008	3	.003	2	.002	3	4.886e-5	3	NC	4	NC	1
350			min	-.009	2	-.003	3	-.002	1	-9.337e-5	1	1796.545	3	NC	1
351		5	max	.008	3	.009	2	.001	3	5.018e-5	3	NC	4	NC	1
352			min	-.009	2	-.009	3	-.003	1	-7.377e-5	1	1450.784	3	NC	1
353		6	max	.008	3	.015	2	.001	3	5.15e-5	3	NC	4	NC	1
354			min	-.009	2	-.014	3	-.002	1	-5.72e-5	9	1257.795	3	NC	1
355		7	max	.008	3	.019	2	.001	3	5.281e-5	3	NC	4	NC	1
356			min	-.008	2	-.018	3	-.002	1	-4.179e-5	9	1143.548	3	NC	1
357		8	max	.008	3	.022	2	0	3	5.413e-5	3	NC	4	NC	1
358			min	-.008	2	-.021	3	-.002	9	-2.638e-5	9	1057.31	2	NC	1
359		9	max	.008	3	.024	2	0	3	5.544e-5	3	NC	4	NC	1
360			min	-.008	2	-.022	3	-.001	9	-1.098e-5	9	1003.529	2	NC	1
361		10	max	.008	3	.025	2	0	3	5.676e-5	3	NC	4	NC	1
362			min	-.008	2	-.023	3	0	9	-9.6e-7	10	977.808	2	NC	1
363		11	max	.008	3	.024	2	.001	3	5.807e-5	3	NC	4	NC	1
364			min	-.008	2	-.022	3	0	9	-2.054e-6	10	977.197	2	NC	1
365		12	max	.008	3	.023	2	.001	3	6.341e-5	1	NC	4	NC	1
366			min	-.008	2	-.02	3	0	10	-3.147e-6	10	1002.711	2	NC	1
367		13	max	.008	3	.02	2	.002	1	8.301e-5	1	NC	4	NC	1
368			min	-.008	2	-.017	3	0	10	-4.241e-6	10	1059.928	2	NC	1
369		14	max	.008	3	.016	2	.002	1	1.026e-4	1	NC	4	NC	1
370			min	-.008	2	-.014	3	0	10	-5.335e-6	10	1161.845	2	NC	1
371		15	max	.008	3	.01	2	.002	1	1.222e-4	1	NC	4	NC	1
372			min	-.008	2	-.009	3	0	10	-6.428e-6	10	1337.12	2	NC	1
373		16	max	.008	3	.003	2	.002	1	1.365e-4	1	NC	4	NC	1
374			min	-.008	2	-.003	3	0	10	-7.238e-6	10	1656.156	2	NC	1
375		17	max	.008	3	.004	3	.002	1	7.2e-5	3	NC	4	NC	1
376			min	-.008	2	-.006	2	0	10	-4.303e-6	9	2346.669	2	NC	1
377		18	max	.008	3	.012	3	.001	3	3.74e-3	2	NC	4	NC	1
378			min	-.008	2	-.016	2	0	10	-2.006e-3	3	4548.509	2	NC	1
379		19	max	.008	3	.02	3	0	3	7.544e-3	2	NC	1	NC	1
380			min	-.008	2	-.027	2	0	9	-4.124e-3	3	NC	1	NC	1
381	M5	1	max	.024	3	.074	3	.004	3	9.789e-6	3	NC	1	NC	1
382			min	-.027	2	-.062	2	0	9	0	1	NC	1	NC	1
383		2	max	.024	3	.044	3	.005	3	1.369e-4	3	NC	4	NC	1
384			min	-.027	2	-.036	2	0	9	-1.182e-5	9	1576.198	3	NC	1
385		3	max	.024	3	.015	3	.006	3	2.615e-4	3	NC	5	NC	1
386			min	-.027	2	-.011	2	0	9	-2.352e-5	9	816.958	3	NC	1
387		4	max	.024	3	.01	2	.007	3	2.526e-4	3	NC	5	NC	1
388			min	-.027	2	-.009	3	0	9	-2.237e-5	9	582.924	3	NC	1
389		5	max	.024	3	.029	2	.008	3	2.437e-4	3	NC	5	NC	1
390			min	-.027	2	-.028	3	0	9	-2.122e-5	9	471.472	3	9416.324	3
391		6	max	.024	3	.046	2	.008	3	2.348e-4	3	NC	5	NC	1
392			min	-.027	2	-.044	3	0	9	-2.007e-5	9	409.402	3	8505.213	3
393		7	max	.024	3	.059	2	.009	3	2.259e-4	3	NC	5	NC	1





Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.008	3	.004	3	0	10	8.522e-5	3	NC	4	NC	1
452		min	-.008	2	-.006	2	-.002	1	-5.632e-5	9	2347.344	2	NC	1
453	18	max	.008	3	.012	3	0	10	2.086e-3	3	NC	4	NC	1
454		min	-.008	2	-.016	2	-.001	1	-3.74e-3	2	4549.776	2	NC	1
455	19	max	.008	3	.02	3	0	3	4.121e-3	3	NC	1	NC	1
456		min	-.008	2	-.027	2	0	9	-7.544e-3	2	NC	1	NC	1
457	M13	1	max	0	.023	3	.008	3	3.709e-3	3	NC	1	NC	1
458		min	-.003	3	-.02	2	-.008	2	-3.17e-3	2	NC	1	NC	1
459	2	max	0	9	.06	3	.007	3	4.579e-3	3	NC	4	NC	1
460		min	-.003	3	-.045	2	-.008	2	-3.918e-3	2	2436.582	3	NC	1
461	3	max	0	9	.092	3	.007	3	5.449e-3	3	NC	4	NC	1
462		min	-.003	3	-.067	2	-.008	2	-4.666e-3	2	1318.981	3	NC	1
463	4	max	0	9	.113	3	.009	9	6.319e-3	3	NC	4	NC	2
464		min	-.003	3	-.082	2	-.008	2	-5.414e-3	2	999.614	3	8362.534	1
465	5	max	0	9	.124	3	.01	3	7.189e-3	3	NC	5	NC	2
466		min	-.003	3	-.091	2	-.01	2	-6.161e-3	2	896.2	3	8144.091	1
467	6	max	0	9	.123	3	.012	3	8.059e-3	3	NC	5	NC	1
468		min	-.004	3	-.091	2	-.014	2	-6.909e-3	2	905.132	3	NC	1
469	7	max	0	9	.113	3	.015	3	8.93e-3	3	NC	4	NC	1
470		min	-.004	3	-.086	2	-.017	2	-7.657e-3	2	1010.822	3	NC	1
471	8	max	0	9	.097	3	.018	3	9.8e-3	3	NC	4	NC	1
472		min	-.004	3	-.076	2	-.022	2	-8.405e-3	2	1230.292	3	6829.559	2
473	9	max	0	9	.081	3	.021	3	1.067e-2	3	NC	4	NC	4
474		min	-.004	3	-.067	2	-.025	2	-9.153e-3	2	1561.228	3	5406.966	2
475	10	max	0	9	.074	3	.024	3	1.154e-2	3	NC	4	NC	4
476		min	-.004	3	-.062	2	-.027	2	-9.901e-3	2	1788.391	3	4962.154	2
477	11	max	0	9	.081	3	.026	3	1.067e-2	3	NC	4	NC	4
478		min	-.004	3	-.067	2	-.025	2	-9.153e-3	2	1561.226	3	5160.063	3
479	12	max	0	9	.097	3	.026	3	9.805e-3	3	NC	4	NC	1
480		min	-.004	3	-.076	2	-.022	2	-8.405e-3	2	1230.29	3	5082.313	3
481	13	max	0	9	.113	3	.025	3	8.937e-3	3	NC	4	NC	1
482		min	-.004	3	-.086	2	-.017	2	-7.657e-3	2	1010.821	3	5383.581	3
483	14	max	0	9	.123	3	.023	3	8.07e-3	3	NC	5	NC	1
484		min	-.004	3	-.091	2	-.014	2	-6.91e-3	2	905.131	3	6125.547	3
485	15	max	0	9	.124	3	.02	3	7.203e-3	3	NC	5	NC	2
486		min	-.004	3	-.09	2	-.01	2	-6.162e-3	2	896.199	3	7561.815	3
487	16	max	0	9	.114	3	.017	3	6.335e-3	3	NC	4	NC	2
488		min	-.004	3	-.082	2	-.008	2	-5.414e-3	2	999.613	3	8365.544	1
489	17	max	0	9	.092	3	.014	3	5.468e-3	3	NC	4	NC	1
490		min	-.004	3	-.067	2	-.008	2	-4.666e-3	2	1318.98	3	NC	1
491	18	max	0	9	.061	3	.011	3	4.6e-3	3	NC	4	NC	1
492		min	-.004	3	-.045	2	-.008	2	-3.918e-3	2	2436.58	3	NC	1
493	19	max	0	9	.024	3	.008	3	3.733e-3	3	NC	1	NC	1
494		min	-.004	3	-.02	2	-.009	2	-3.17e-3	2	NC	1	NC	1
495	M16	1	max	0	.02	3	.008	3	4.047e-3	2	NC	1	NC	1
496		min	0	3	-.027	2	-.008	2	-3.045e-3	3	NC	1	NC	1
497	2	max	0	9	.04	3	.011	3	5.005e-3	2	NC	4	NC	1
498		min	0	3	-.063	2	-.008	2	-3.721e-3	3	2459.873	2	NC	1
499	3	max	0	9	.057	3	.013	3	5.963e-3	2	NC	4	NC	1
500		min	0	3	-.094	2	-.008	2	-4.398e-3	3	1327.838	2	NC	1
501	4	max	0	9	.07	3	.016	3	6.921e-3	2	NC	4	NC	2
502		min	0	3	-.117	2	-.008	2	-5.074e-3	3	1001.403	2	8413.432	1
503	5	max	0	9	.077	3	.019	3	7.879e-3	2	NC	5	NC	2
504		min	0	3	-.128	2	-.01	2	-5.751e-3	3	890.949	2	8132.326	3
505	6	max	0	9	.079	3	.021	3	8.838e-3	2	NC	5	NC	1
506		min	0	3	-.128	2	-.013	2	-6.427e-3	3	889.353	2	6755.952	3
507	7	max	0	9	.076	3	.023	3	9.796e-3	2	NC	4	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.119	2	-.017	2	-7.104e-3	3	975.531	2	6014.191	3
509	8	max	0	9	.07	3	.024	3	1.075e-2	2	NC	4	NC	1
510		min	0	3	-.105	2	-.022	2	-7.78e-3	3	1155.578	2	5670.974	3
511	9	max	0	9	.063	3	.024	3	1.171e-2	2	NC	4	NC	4
512		min	0	3	-.09	2	-.025	2	-8.457e-3	3	1414.915	2	5427.585	2
513	10	max	0	9	.06	3	.023	3	1.267e-2	2	NC	4	NC	4
514		min	0	3	-.083	2	-.027	2	-9.133e-3	3	1584.129	2	4980.074	2
515	11	max	0	9	.063	3	.022	3	1.171e-2	2	NC	4	NC	4
516		min	0	3	-.09	2	-.025	2	-8.454e-3	3	1414.915	2	5427.592	2
517	12	max	0	9	.07	3	.02	3	1.075e-2	2	NC	4	NC	1
518		min	0	3	-.105	2	-.022	2	-7.775e-3	3	1155.578	2	6859.653	2
519	13	max	0	9	.076	3	.019	3	9.796e-3	2	NC	4	NC	1
520		min	0	3	-.119	2	-.017	2	-7.096e-3	3	975.531	2	8404.868	3
521	14	max	0	9	.079	3	.017	3	8.838e-3	2	NC	5	NC	1
522		min	0	3	-.128	2	-.013	2	-6.417e-3	3	889.353	2	NC	1
523	15	max	0	9	.077	3	.015	3	7.88e-3	2	NC	5	NC	2
524		min	0	3	-.128	2	-.01	2	-5.738e-3	3	890.949	2	8210.927	1
525	16	max	0	9	.07	3	.013	3	6.922e-3	2	NC	4	NC	2
526		min	0	3	-.117	2	-.008	2	-5.059e-3	3	1001.403	2	8422.704	1
527	17	max	0	9	.057	3	.011	3	5.964e-3	2	NC	4	NC	1
528		min	0	3	-.094	2	-.008	2	-4.38e-3	3	1327.838	2	NC	1
529	18	max	0	9	.04	3	.009	3	5.006e-3	2	NC	4	NC	1
530		min	0	3	-.063	2	-.008	2	-3.701e-3	3	2459.873	2	NC	1
531	19	max	0	9	.02	3	.008	3	4.048e-3	2	NC	1	NC	1
532		min	0	3	-.027	2	-.008	2	-3.023e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.945e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.088e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.716e-4	3	NC	1	NC	1
536		min	0	2	-.002	4	0	3	-4.106e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	1	1.149e-3	3	NC	1	NC	1
538		min	0	2	-.005	4	-.003	3	-7.803e-4	2	NC	1	9428.373	3
539	4	max	0	3	-.002	15	.005	2	1.526e-3	3	NC	1	NC	4
540		min	0	2	-.007	4	-.007	3	-1.15e-3	2	8957.242	4	5219.556	3
541	5	max	0	3	-.002	15	.009	2	1.903e-3	3	NC	3	NC	4
542		min	0	2	-.009	4	-.011	3	-1.52e-3	2	6989.427	4	3433.721	3
543	6	max	0	3	-.002	15	.013	2	2.28e-3	3	NC	3	NC	4
544		min	-.001	2	-.01	4	-.016	3	-1.889e-3	2	5882.342	4	2504.189	3
545	7	max	0	3	-.003	15	.016	2	2.657e-3	3	NC	5	NC	4
546		min	-.001	2	-.012	4	-.021	3	-2.259e-3	2	5216.576	4	1959.886	3
547	8	max	0	3	-.003	15	.02	2	3.034e-3	3	NC	5	NC	4
548		min	-.002	2	-.013	4	-.026	3	-2.629e-3	2	4817.016	4	1617.39	3
549	9	max	0	3	-.003	15	.024	2	3.411e-3	3	NC	5	NC	4
550		min	-.002	2	-.013	4	-.03	3	-2.999e-3	2	4601.949	4	1393.124	3
551	10	max	0	3	-.003	15	.026	2	3.788e-3	3	NC	5	NC	4
552		min	-.002	2	-.013	4	-.033	3	-3.368e-3	2	4533.913	4	1245.068	3
553	11	max	.001	3	-.003	15	.028	2	4.166e-3	3	NC	5	NC	4
554		min	-.002	2	-.013	4	-.036	3	-3.738e-3	2	4601.949	4	1151.151	3
555	12	max	.001	3	-.003	15	.028	2	4.543e-3	3	NC	5	NC	4
556		min	-.002	2	-.013	4	-.036	3	-4.108e-3	2	4817.016	4	1100.433	3
557	13	max	.001	3	-.003	15	.027	2	4.92e-3	3	NC	5	NC	4
558		min	-.003	2	-.012	4	-.035	3	-4.477e-3	2	5216.576	4	1089.773	3
559	14	max	.001	3	-.002	15	.024	2	5.297e-3	3	NC	3	NC	4
560		min	-.003	2	-.01	4	-.032	3	-4.847e-3	2	5882.342	4	1123.967	3
561	15	max	.001	3	-.001	2	.019	1	5.674e-3	3	NC	3	NC	4
562		min	-.003	2	-.009	4	-.026	3	-5.217e-3	2	6989.427	4	1220.516	3
563	16	max	.002	3	0	2	.014	1	6.051e-3	3	NC	1	NC	4
564		min	-.003	2	-.007	4	-.017	3	-5.586e-3	2	8957.242	4	1426.894	3

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.002	3	.003	2	.005	1	6.428e-3	3	NC	1	NC	4
566			min	-.003	2	-.005	4	-.006	3	-5.956e-3	2	NC	1	1892.005	3
567		18	max	.002	3	.005	2	.01	3	6.805e-3	3	NC	1	NC	4
568			min	-.004	2	-.003	4	-.012	2	-6.326e-3	2	NC	1	3369.064	3
569		19	max	.002	3	.007	2	.029	3	7.182e-3	3	NC	1	NC	1
570			min	-.004	2	0	9	-.028	2	-6.696e-3	2	NC	1	NC	1
571	M16A	1	max	.001	2	.002	2	.009	3	2.042e-3	3	NC	1	NC	1
572			min	-.002	3	-.002	3	-.009	2	-2.114e-3	2	NC	1	NC	1
573		2	max	0	2	0	2	.002	3	1.967e-3	3	NC	1	NC	1
574			min	-.002	3	-.003	3	-.004	2	-2.017e-3	2	NC	1	9360.123	3
575		3	max	0	2	-.001	15	.003	1	1.892e-3	3	NC	1	NC	4
576			min	-.002	3	-.005	4	-.003	3	-1.921e-3	2	NC	1	5301.72	3
577		4	max	0	2	-.002	15	.005	1	1.816e-3	3	NC	1	NC	4
578			min	-.002	3	-.007	4	-.007	3	-1.824e-3	2	8957.242	4	4037.362	3
579		5	max	0	2	-.002	15	.007	1	1.741e-3	3	NC	3	NC	4
580			min	-.001	3	-.009	4	-.01	3	-1.728e-3	2	6989.427	4	3491.832	3
581		6	max	0	2	-.002	15	.008	1	1.666e-3	3	NC	3	NC	4
582			min	-.001	3	-.01	4	-.012	3	-1.631e-3	2	5882.342	4	3256.826	3
583		7	max	0	2	-.003	15	.009	1	1.591e-3	3	NC	5	NC	4
584			min	-.001	3	-.012	4	-.013	3	-1.535e-3	2	5216.576	4	3204.903	3
585		8	max	0	2	-.003	15	.009	1	1.516e-3	3	NC	5	NC	4
586			min	-.001	3	-.013	4	-.013	3	-1.439e-3	2	4817.016	4	3293.297	3
587		9	max	0	2	-.003	15	.009	1	1.441e-3	3	NC	5	NC	4
588			min	-.001	3	-.013	4	-.012	3	-1.342e-3	2	4601.949	4	3517.857	3
589		10	max	0	2	-.003	15	.008	1	1.366e-3	3	NC	5	NC	4
590			min	0	3	-.013	4	-.011	3	-1.246e-3	2	4533.913	4	3902.993	3
591		11	max	0	2	-.003	15	.007	1	1.291e-3	3	NC	5	NC	4
592			min	0	3	-.013	4	-.009	3	-1.149e-3	2	4601.949	4	4507.815	3
593		12	max	0	2	-.003	15	.005	1	1.216e-3	3	NC	5	NC	4
594			min	0	3	-.013	4	-.007	3	-1.053e-3	2	4817.016	4	5450.332	3
595		13	max	0	2	-.003	15	.004	1	1.141e-3	3	NC	5	NC	2
596			min	0	3	-.012	4	-.006	3	-9.564e-4	2	5216.576	4	6970.15	3
597		14	max	0	2	-.002	15	.003	1	1.065e-3	3	NC	3	NC	1
598			min	0	3	-.01	4	-.004	3	-.8.6e-4	2	5882.342	4	9600.653	3
599		15	max	0	2	-.002	15	.002	1	9.903e-4	3	NC	3	NC	1
600			min	0	3	-.009	4	-.002	3	-7.635e-4	2	6989.427	4	NC	1
601		16	max	0	2	-.002	15	.001	4	9.152e-4	3	NC	1	NC	1
602			min	0	3	-.007	4	0	3	-6.671e-4	2	8957.242	4	NC	1
603		17	max	0	2	-.001	15	0	4	8.401e-4	3	NC	1	NC	1
604			min	0	3	-.005	4	0	2	-5.707e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	3	7.65e-4	3	NC	1	NC	1
606			min	0	3	-.002	4	0	2	-4.742e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	6.899e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.778e-4	2	NC	1	NC	1



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
 Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.