

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

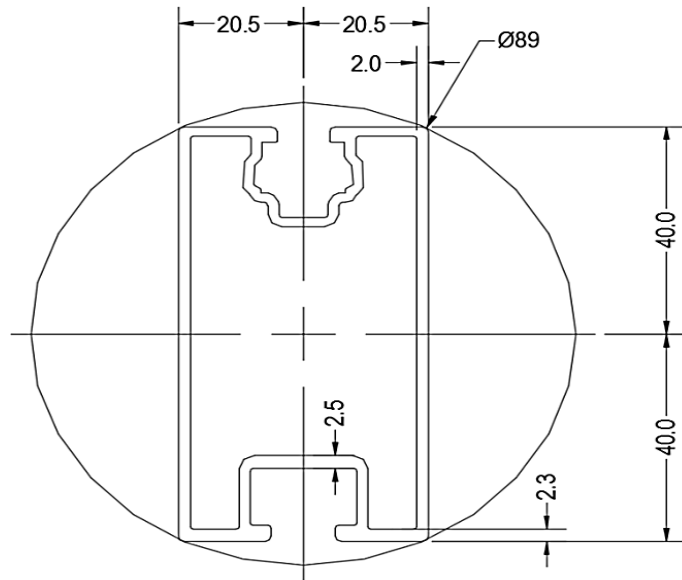
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	45 in
ΦF_{ty} STRONG-AXIS =	30.25 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.304 k-ft
M_z =	0.045 k-ft
$M_{y \text{ allowable}}$ =	1.879 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	22%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.93 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.427 k-ft
M_z =	-0.015 k-ft
P_n =	-0.043 k
$M_{y \text{ allowable}}$ =	1.469 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	29.696 k
Utilization =	32%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.025 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.181 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	5%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.748 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	9%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.156 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 35 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

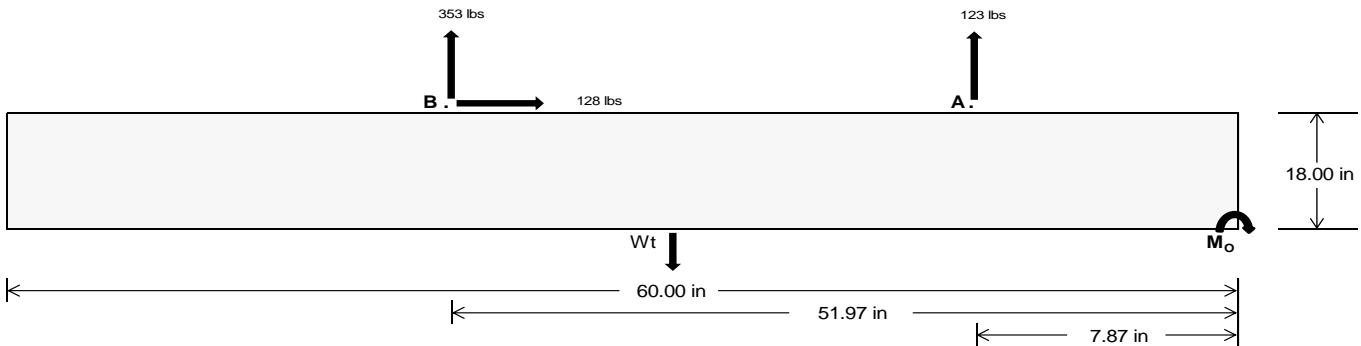
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	514.21	1469.96	k
Compressive Load =	1332.11	949.61	k
Lateral Load =	19.27	532.81	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21613.5$ in-lbs
Resisting Force Required = 720.45 lbs
S.F. = 1.67
Weight Required = 1200.75 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 128.09 lbs
Friction = 0.4
Weight Required = 320.23 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 128.09 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	391 lbs	391 lbs	391 lbs	391 lbs	550 lbs	550 lbs	550 lbs	550 lbs	678 lbs	678 lbs	678 lbs	678 lbs	-246 lbs	-246 lbs	-246 lbs	-246 lbs
F_B	278 lbs	278 lbs	278 lbs	278 lbs	393 lbs	393 lbs	393 lbs	393 lbs	482 lbs	482 lbs	482 lbs	482 lbs	-706 lbs	-706 lbs	-706 lbs	-706 lbs
F_V	16 lbs	16 lbs	16 lbs	16 lbs	221 lbs	221 lbs	221 lbs	221 lbs	177 lbs	177 lbs	177 lbs	177 lbs	-256 lbs	-256 lbs	-256 lbs	-256 lbs
P_{total}	2572 lbs	2662 lbs	2753 lbs	2844 lbs	2846 lbs	2936 lbs	3027 lbs	3118 lbs	3063 lbs	3154 lbs	3244 lbs	3335 lbs	190 lbs	245 lbs	299 lbs	353 lbs
M	236 lbs-ft	236 lbs-ft	236 lbs-ft	236 lbs-ft	626 lbs-ft	626 lbs-ft	626 lbs-ft	626 lbs-ft	632 lbs-ft	632 lbs-ft	632 lbs-ft	632 lbs-ft	455 lbs-ft	455 lbs-ft	455 lbs-ft	455 lbs-ft
e	0.09 ft	0.09 ft	0.09 ft	0.08 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.21 ft	0.20 ft	0.19 ft	0.19 ft	2.39 ft	1.86 ft	1.52 ft	1.29 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	261.6 psf	259.6 psf	257.8 psf	256.1 psf	239.4 psf	238.4 psf	237.5 psf	236.6 psf	263.5 psf	261.4 psf	259.5 psf	257.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	326.2 psf	321.3 psf	316.8 psf	312.6 psf	411.1 psf	402.3 psf	394.3 psf	386.9 psf	436.7 psf	426.7 psf	417.6 psf	409.3 psf	657.3 psf	138.7 psf	106.2 psf	97.1 psf

Maximum Bearing Pressure = 657 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

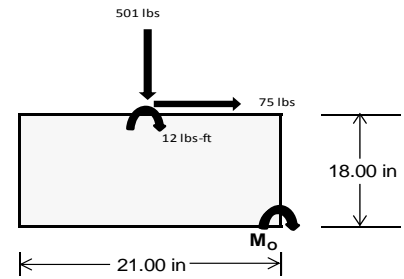
Overturning Check

$M_o = 314.1 \text{ ft-lbs}$
 Resisting Force Required = 359.02 lbs
 S.F. = 1.67
 Weight Required = 598.36 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	102 lbs	64 lbs	50 lbs	228 lbs	501 lbs	188 lbs	67 lbs	-21 lbs	17 lbs
F_v	11 lbs	99 lbs	11 lbs	8 lbs	75 lbs	8 lbs	11 lbs	99 lbs	11 lbs
P_{total}	2458 lbs	2420 lbs	2406 lbs	2471 lbs	2743 lbs	2431 lbs	756 lbs	668 lbs	706 lbs
M	31 lbs-ft	165 lbs-ft	32 lbs-ft	23 lbs-ft	124 lbs-ft	25 lbs-ft	31 lbs-ft	165 lbs-ft	32 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.25 ft	0.04 ft
$L/6$	0.29 ft	1.61 ft	1.72 ft	1.73 ft	1.66 ft	1.73 ft	1.67 ft	1.26 ft	1.66 ft
f_{min}	268.8 sqft	211.9 sqft	262.5 sqft	273.2 sqft	265.0 sqft	268.1 sqft	74.3 sqft	11.7 sqft	68.3 sqft
f_{max}	293.0 psf	341.2 psf	287.4 psf	291.5 psf	362.1 psf	287.5 psf	98.5 psf	140.9 psf	93.1 psf



Maximum Bearing Pressure = 362 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

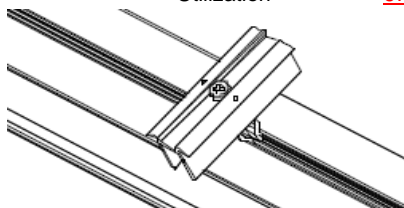
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

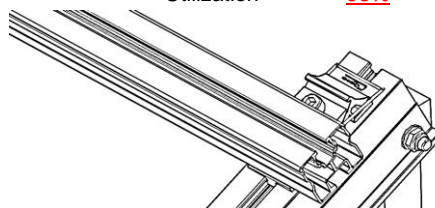
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.817 k
Allowable Uplift =	1.214 k
Utilization =	<u>67%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.092 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

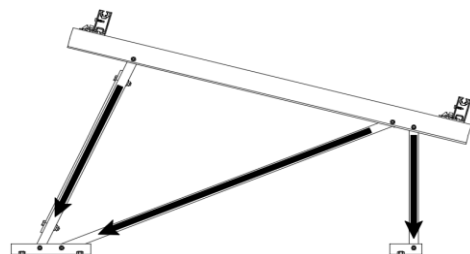
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.025 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Diagonal Strut

Maximum Axial Load =	0.181 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.106 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.156 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	$0.020h_{sx}$
	0.568 in
Max Drift, Δ_{MAX} =	0.048 in
	<u>$0.048 \leq 0.568$. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.427$$

$$93.8539$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.427$$

$$101.986$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.2 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.879 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.45 \\ &20.4426 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.45 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.469 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 15° 130mph 30psf 3.75ft 7-05Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	245.983	1	.132	6	.079	9	0	10	0	4	0	15
30			min	-370.197	3	.028	15	-.44	5	0	4	0	3	0	6
31		16	max	246.08	1	.098	2	.079	9	0	10	0	4	0	15
32			min	-370.125	3	.02	15	-.527	5	0	4	0	3	0	6
33		17	max	246.176	1	.068	2	.079	9	0	10	0	4	0	15
34			min	-370.053	3	.011	15	-.614	5	0	4	0	3	0	6
35		18	max	246.272	1	.039	2	.079	9	0	10	0	14	0	15
36			min	-369.98	3	.002	15	-.702	5	0	4	0	3	0	6
37		19	max	246.369	1	.009	10	.079	9	0	10	0	9	0	15
38			min	-369.908	3	-.022	14	-.789	5	0	4	0	3	0	6
39	M3	1	max	60.643	2	1.812	6	.008	10	0	5	0	4	0	6
40			min	-46.81	14	.424	15	-1.352	4	0	1	0	10	0	15
41		2	max	60.576	2	1.634	6	.008	10	0	5	0	4	0	6
42			min	-46.876	14	.382	15	-1.218	4	0	1	0	10	0	15
43		3	max	60.509	2	1.456	6	.008	10	0	5	0	1	0	2
44			min	-46.941	14	.341	15	-1.085	4	0	1	0	10	0	15
45		4	max	60.442	2	1.278	6	.008	10	0	5	0	1	0	15
46			min	-47.007	14	.299	15	-.951	4	0	1	0	5	0	4
47		5	max	60.375	2	1.1	6	.008	10	0	5	0	1	0	15
48			min	-47.073	14	.257	15	-.818	4	0	1	0	5	0	4
49		6	max	60.308	2	.922	6	.008	10	0	5	0	1	0	15
50			min	-47.139	14	.215	15	-.684	4	0	1	0	5	0	4
51		7	max	60.241	2	.744	6	.008	10	0	5	0	1	0	15
52			min	-47.205	14	.173	15	-.551	4	0	1	0	5	0	4
53		8	max	60.173	2	.566	6	.008	10	0	5	0	1	0	15
54			min	-47.271	14	.131	15	-.417	4	0	1	0	5	0	4
55		9	max	60.106	2	.388	6	.008	10	0	5	0	1	0	15
56			min	-47.337	14	.09	15	-.284	4	0	1	0	5	-.001	4
57		10	max	60.039	2	.21	6	.008	10	0	5	0	1	0	15
58			min	-47.403	14	.048	15	-.15	4	0	1	0	5	-.001	4
59		11	max	59.972	2	.041	2	.008	5	0	5	0	1	0	15
60			min	-47.469	14	.006	15	-.111	1	0	1	0	5	-.001	4
61		12	max	59.905	2	-.036	15	.141	5	0	5	0	1	0	15
62			min	-47.535	14	-.146	4	-.111	1	0	1	0	5	-.001	4
63		13	max	59.838	2	-.078	15	.275	5	0	5	0	9	0	15
64			min	-47.601	14	-.324	4	-.111	1	0	1	0	5	-.001	4
65		14	max	59.771	2	-.12	15	.408	5	0	5	0	3	0	15
66			min	-47.667	14	-.502	4	-.111	1	0	1	0	5	-.001	4
67		15	max	59.704	2	-.162	15	.542	5	0	5	0	10	0	15
68			min	-47.733	14	-.68	4	-.111	1	0	1	0	4	0	4
69		16	max	59.637	2	-.203	15	.676	5	0	5	0	10	0	15
70			min	-47.798	14	-.858	4	-.111	1	0	1	0	4	0	4
71		17	max	59.57	2	-.245	15	.809	5	0	5	0	10	0	15
72			min	-47.864	14	-1.036	4	-.111	1	0	1	0	4	0	4
73		18	max	59.502	2	-.287	15	.943	5	0	5	0	10	0	15
74			min	-47.93	14	-1.214	4	-.111	1	0	1	0	4	0	4
75		19	max	59.435	2	-.329	15	1.076	5	0	5	0	5	0	1
76			min	-47.996	14	-1.392	4	-.111	1	0	1	0	1	0	1
77	M4	1	max	338.281	1	0	1	.03	10	0	1	0	5	0	1
78			min	-120.456	3	0	1	-13.59	4	0	1	0	2	0	1
79		2	max	338.346	1	0	1	.03	10	0	1	0	10	0	1
80			min	-120.407	3	0	1	-13.646	4	0	1	-.001	4	0	1
81		3	max	338.41	1	0	1	.03	10	0	1	0	10	0	1
82			min	-120.359	3	0	1	-13.702	4	0	1	-.002	4	0	1
83		4	max	338.475	1	0	1	.03	10	0	1	0	10	0	1
84			min	-120.31	3	0	1	-13.758	4	0	1	-.004	4	0	1
85		5	max	338.54	1	0	1	.03	10	0	1	0	10	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-120.262	3	0	1	-13.814	4	0	1	-.005	4	0	1
87		6	max	338.605	1	0	1	.03	10	0	1	0	10	0	1
88			min	-120.213	3	0	1	-13.87	4	0	1	-.006	4	0	1
89		7	max	338.669	1	0	1	.03	10	0	1	0	10	0	1
90			min	-120.165	3	0	1	-13.926	4	0	1	-.007	4	0	1
91		8	max	338.734	1	0	1	.03	10	0	1	0	10	0	1
92			min	-120.116	3	0	1	-13.982	4	0	1	-.009	4	0	1
93		9	max	338.799	1	0	1	.03	10	0	1	0	10	0	1
94			min	-120.068	3	0	1	-14.038	4	0	1	-.01	4	0	1
95		10	max	338.863	1	0	1	.03	10	0	1	0	10	0	1
96			min	-120.019	3	0	1	-14.094	4	0	1	-.011	4	0	1
97		11	max	338.928	1	0	1	.03	10	0	1	0	10	0	1
98			min	-119.971	3	0	1	-14.15	4	0	1	-.012	4	0	1
99		12	max	338.993	1	0	1	.03	10	0	1	0	10	0	1
100			min	-119.922	3	0	1	-14.207	4	0	1	-.014	4	0	1
101		13	max	339.058	1	0	1	.03	10	0	1	0	10	0	1
102			min	-119.874	3	0	1	-14.263	4	0	1	-.015	4	0	1
103		14	max	339.122	1	0	1	.03	10	0	1	0	10	0	1
104			min	-119.825	3	0	1	-14.319	4	0	1	-.016	4	0	1
105		15	max	339.187	1	0	1	.03	10	0	1	0	10	0	1
106			min	-119.777	3	0	1	-14.375	4	0	1	-.017	4	0	1
107		16	max	339.252	1	0	1	.03	10	0	1	0	10	0	1
108			min	-119.728	3	0	1	-14.431	4	0	1	-.019	4	0	1
109		17	max	339.316	1	0	1	.03	10	0	1	0	10	0	1
110			min	-119.68	3	0	1	-14.487	4	0	1	-.02	4	0	1
111		18	max	339.381	1	0	1	.03	10	0	1	0	10	0	1
112			min	-119.631	3	0	1	-14.543	4	0	1	-.021	4	0	1
113		19	max	339.446	1	0	1	.03	10	0	1	0	10	0	1
114			min	-119.582	3	0	1	-14.599	4	0	1	-.023	4	0	1
115	M6	1	max	746.373	1	.649	6	.807	4	0	3	0	3	0	1
116			min	-1105.77	3	.15	15	-.291	3	0	5	0	9	0	1
117		2	max	746.469	1	.611	6	.72	4	0	3	0	4	0	15
118			min	-1105.698	3	.141	15	-.291	3	0	5	0	1	0	6
119		3	max	746.565	1	.574	6	.632	4	0	3	0	4	0	15
120			min	-1105.626	3	.132	15	-.291	3	0	5	0	1	0	6
121		4	max	746.662	1	.536	6	.545	4	0	3	0	4	0	15
122			min	-1105.553	3	.123	15	-.291	3	0	5	0	1	0	6
123		5	max	746.758	1	.498	6	.458	4	0	3	0	4	0	15
124			min	-1105.481	3	.115	15	-.291	3	0	5	0	3	0	6
125		6	max	746.855	1	.46	6	.37	4	0	3	0	4	0	15
126			min	-1105.409	3	.106	15	-.291	3	0	5	0	3	0	6
127		7	max	746.951	1	.422	6	.283	4	0	3	0	4	0	15
128			min	-1105.337	3	.097	15	-.291	3	0	5	0	3	0	6
129		8	max	747.047	1	.384	6	.196	4	0	3	0	4	0	15
130			min	-1105.264	3	.088	15	-.291	3	0	5	0	3	0	6
131		9	max	747.144	1	.347	6	.108	4	0	3	0	4	0	15
132			min	-1105.192	3	.079	15	-.291	3	0	5	0	3	0	6
133		10	max	747.24	1	.309	6	.023	14	0	3	0	4	0	15
134			min	-1105.12	3	.07	15	-.291	3	0	5	0	3	0	6
135		11	max	747.336	1	.271	6	.016	9	0	3	0	4	0	15
136			min	-1105.048	3	.061	15	-.291	3	0	5	0	3	0	6
137		12	max	747.433	1	.24	2	.016	9	0	3	0	4	0	15
138			min	-1104.975	3	.052	15	-.291	3	0	5	0	3	0	6
139		13	max	747.529	1	.21	2	.016	9	0	3	0	4	0	15
140			min	-1104.903	3	.043	15	-.291	3	0	5	0	3	0	6
141		14	max	747.625	1	.181	2	.016	9	0	3	0	4	0	15
142			min	-1104.831	3	.035	15	-.335	5	0	5	0	3	0	6



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	747.722	1	.151	2	.016	9	0	3	0	4	0	15
144		min	-1104.758	3	.026	15	-.423	5	0	5	0	3	0	6
145	16	max	747.818	1	.122	2	.016	9	0	3	0	4	0	15
146		min	-1104.686	3	.017	15	-.51	5	0	5	0	3	0	6
147	17	max	747.915	1	.092	2	.016	9	0	3	0	4	0	15
148		min	-1104.614	3	.003	9	-.597	5	0	5	0	3	0	6
149	18	max	748.011	1	.063	2	.016	9	0	3	0	4	0	15
150		min	-1104.542	3	-.022	9	-.685	5	0	5	0	3	0	6
151	19	max	748.107	1	.033	2	.016	9	0	3	0	4	0	15
152		min	-1104.469	3	-.046	9	-.772	5	0	5	0	3	0	6
153	M7	1	max	180.976	2	1.82	.007	3	0	9	0	4	0	4
154		min	-88.521	9	.432	15	-1.417	4	0	3	0	3	0	15
155	2	max	180.909	2	1.642	4	.007	3	0	9	0	4	0	4
156		min	-88.577	9	.39	15	-1.283	4	0	3	0	3	0	15
157	3	max	180.842	2	1.464	4	.007	3	0	9	0	4	0	2
158		min	-88.633	9	.348	15	-1.149	4	0	3	0	3	0	9
159	4	max	180.775	2	1.286	4	.007	3	0	9	0	9	0	15
160		min	-88.688	9	.306	15	-1.016	4	0	3	0	3	0	1
161	5	max	180.707	2	1.108	4	.007	3	0	9	0	9	0	15
162		min	-88.744	9	.264	15	-.882	4	0	3	0	3	0	6
163	6	max	180.64	2	.93	4	.007	3	0	9	0	9	0	15
164		min	-88.8	9	.222	15	-.749	4	0	3	0	5	0	6
165	7	max	180.573	2	.752	4	.007	3	0	9	0	9	0	15
166		min	-88.856	9	.181	15	-.615	4	0	3	0	5	0	6
167	8	max	180.506	2	.574	4	.007	3	0	9	0	9	0	15
168		min	-88.912	9	.139	15	-.482	4	0	3	0	5	0	6
169	9	max	180.439	2	.396	4	.007	3	0	9	0	9	0	15
170		min	-88.968	9	.097	15	-.348	4	0	3	0	5	-.001	6
171	10	max	180.372	2	.218	4	.007	3	0	9	0	9	0	15
172		min	-89.024	9	.055	15	-.215	4	0	3	0	5	-.001	6
173	11	max	180.305	2	.057	2	.007	3	0	9	0	9	0	15
174		min	-89.08	9	.003	9	-.081	4	0	3	0	5	-.001	6
175	12	max	180.238	2	-.029	15	.055	5	0	9	0	9	0	15
176		min	-89.136	9	-.139	6	-.006	9	0	3	0	5	-.001	6
177	13	max	180.171	2	-.07	15	.189	5	0	9	0	9	0	15
178		min	-89.192	9	-.317	6	-.006	9	0	3	0	5	-.001	6
179	14	max	180.104	2	-.112	15	.322	5	0	9	0	9	0	15
180		min	-89.248	9	-.495	6	-.006	9	0	3	0	5	-.001	6
181	15	max	180.036	2	-.154	15	.456	5	0	9	0	9	0	15
182		min	-89.303	9	-.673	6	-.006	9	0	3	0	5	0	6
183	16	max	179.969	2	-.196	15	.59	5	0	9	0	9	0	15
184		min	-89.359	9	-.851	6	-.006	9	0	3	0	5	0	6
185	17	max	179.902	2	-.238	15	.723	5	0	9	0	9	0	15
186		min	-89.415	9	-1.029	6	-.006	9	0	3	0	5	0	6
187	18	max	179.835	2	-.28	15	.857	5	0	9	0	9	0	15
188		min	-89.471	9	-1.207	6	-.006	9	0	3	0	5	0	6
189	19	max	179.768	2	-.322	15	.99	5	0	9	0	9	0	1
190		min	-89.527	9	-1.385	6	-.006	9	0	3	0	3	0	1
191	M8	1	max	1023.537	2	0	.06	9	0	1	0	4	0	1
192		min	-396.423	3	0	1	-13.937	4	0	1	0	3	0	1
193	2	max	1023.602	2	0	1	.06	9	0	1	0	9	0	1
194		min	-396.374	3	0	1	-13.993	4	0	1	-.001	4	0	1
195	3	max	1023.667	2	0	1	.06	9	0	1	0	9	0	1
196		min	-396.326	3	0	1	-14.049	4	0	1	-.002	4	0	1
197	4	max	1023.732	2	0	1	.06	9	0	1	0	9	0	1
198		min	-396.277	3	0	1	-14.105	4	0	1	-.004	4	0	1
199	5	max	1023.796	2	0	1	.06	9	0	1	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-396.228	3	0	1	-14.161	4	0	1	-.005	4	0	1
201		6	max	1023.861	2	0	1	.06	9	0	1	0	9	0	1
202			min	-396.18	3	0	1	-14.217	4	0	1	-.006	4	0	1
203		7	max	1023.926	2	0	1	.06	9	0	1	0	9	0	1
204			min	-396.131	3	0	1	-14.273	4	0	1	-.008	4	0	1
205		8	max	1023.99	2	0	1	.06	9	0	1	0	9	0	1
206			min	-396.083	3	0	1	-14.329	4	0	1	-.009	4	0	1
207		9	max	1024.055	2	0	1	.06	9	0	1	0	9	0	1
208			min	-396.034	3	0	1	-14.385	4	0	1	-.01	4	0	1
209		10	max	1024.12	2	0	1	.06	9	0	1	0	9	0	1
210			min	-395.986	3	0	1	-14.441	4	0	1	-.011	4	0	1
211		11	max	1024.184	2	0	1	.06	9	0	1	0	9	0	1
212			min	-395.937	3	0	1	-14.497	4	0	1	-.013	4	0	1
213		12	max	1024.249	2	0	1	.06	9	0	1	0	9	0	1
214			min	-395.889	3	0	1	-14.553	4	0	1	-.014	4	0	1
215		13	max	1024.314	2	0	1	.06	9	0	1	0	9	0	1
216			min	-395.84	3	0	1	-14.61	4	0	1	-.015	4	0	1
217		14	max	1024.379	2	0	1	.06	9	0	1	0	9	0	1
218			min	-395.792	3	0	1	-14.666	4	0	1	-.017	4	0	1
219		15	max	1024.443	2	0	1	.06	9	0	1	0	9	0	1
220			min	-395.743	3	0	1	-14.722	4	0	1	-.018	4	0	1
221		16	max	1024.508	2	0	1	.06	9	0	1	0	9	0	1
222			min	-395.695	3	0	1	-14.778	4	0	1	-.019	4	0	1
223		17	max	1024.573	2	0	1	.06	9	0	1	0	9	0	1
224			min	-395.646	3	0	1	-14.834	4	0	1	-.021	4	0	1
225		18	max	1024.637	2	0	1	.06	9	0	1	0	9	0	1
226			min	-395.598	3	0	1	-14.89	4	0	1	-.022	4	0	1
227		19	max	1024.702	2	0	1	.06	9	0	1	0	9	0	1
228			min	-395.549	3	0	1	-14.946	4	0	1	-.023	4	0	1
229	M10	1	max	245.83	1	.693	4	.922	5	0	1	0	4	0	1
230			min	-327.243	3	.174	15	-.089	3	-.001	5	0	3	0	1
231		2	max	245.926	1	.655	4	.835	5	0	1	0	4	0	15
232			min	-327.171	3	.165	15	-.089	3	-.001	5	0	3	0	4
233		3	max	246.022	1	.617	4	.748	5	0	1	0	4	0	15
234			min	-327.098	3	.156	15	-.089	3	-.001	5	0	3	0	4
235		4	max	246.119	1	.579	4	.66	5	0	1	0	4	0	15
236			min	-327.026	3	.147	15	-.089	3	-.001	5	0	3	0	4
237		5	max	246.215	1	.542	4	.573	5	0	1	0	5	0	15
238			min	-326.954	3	.138	15	-.089	3	-.001	5	0	3	0	4
239		6	max	246.312	1	.504	4	.486	5	0	1	0	5	0	15
240			min	-326.882	3	.13	15	-.089	3	-.001	5	0	3	0	4
241		7	max	246.408	1	.466	4	.398	5	0	1	0	5	0	15
242			min	-326.809	3	.121	15	-.089	3	-.001	5	0	3	0	4
243		8	max	246.504	1	.428	4	.311	5	0	1	0	5	0	15
244			min	-326.737	3	.112	15	-.089	3	-.001	5	0	3	0	4
245		9	max	246.601	1	.39	4	.224	5	0	1	0	5	0	15
246			min	-326.665	3	.103	15	-.089	3	-.001	5	0	3	0	4
247		10	max	246.697	1	.353	4	.136	5	0	1	0	5	0	15
248			min	-326.593	3	.094	15	-.089	3	-.001	5	0	3	0	4
249		11	max	246.793	1	.315	4	.049	5	0	1	0	5	0	15
250			min	-326.52	3	.085	15	-.089	3	-.001	5	0	3	0	4
251		12	max	246.89	1	.277	4	.003	10	0	1	0	5	0	15
252			min	-326.448	3	.076	15	-.089	3	-.001	5	0	3	0	4
253		13	max	246.986	1	.239	4	.003	10	0	1	0	5	0	15
254			min	-326.376	3	.067	15	-.138	4	-.001	5	0	3	0	4
255		14	max	247.082	1	.201	4	.003	10	0	1	0	5	0	15
256			min	-326.303	3	.058	15	-.225	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	247.179	1	.163	4	.003	10	0	1	0	5	0	15
258			min	-326.231	3	.05	15	-.312	4	-.001	5	0	3	0	4
259		16	max	247.275	1	.126	4	.003	10	0	1	0	5	0	15
260			min	-326.159	3	.041	15	-.4	4	-.001	5	0	3	0	4
261		17	max	247.372	1	.088	4	.003	10	0	1	0	5	0	15
262			min	-326.087	3	.028	9	-.487	4	-.001	5	0	3	0	4
263		18	max	247.468	1	.05	4	.003	10	0	1	0	5	0	15
264			min	-326.014	3	.003	9	-.574	4	-.001	5	0	3	0	4
265		19	max	247.564	1	.021	3	.003	10	0	1	0	5	0	15
266			min	-325.942	3	-.021	9	-.662	4	-.001	5	0	3	0	4
267	M11	1	max	60.255	2	1.812	6	.111	1	0	4	0	5	0	6
268			min	-44.053	9	.424	15	-1.267	5	0	10	0	1	0	15
269		2	max	60.188	2	1.634	6	.111	1	0	4	0	5	0	6
270			min	-44.108	9	.382	15	-1.133	5	0	10	0	1	0	15
271		3	max	60.121	2	1.456	6	.111	1	0	4	0	5	0	2
272			min	-44.164	9	.34	15	-1	5	0	10	0	1	0	12
273		4	max	60.054	2	1.278	6	.111	1	0	4	0	3	0	15
274			min	-44.22	9	.299	15	-.866	5	0	10	0	1	0	4
275		5	max	59.987	2	1.1	6	.111	1	0	4	0	3	0	15
276			min	-44.276	9	.257	15	-.733	5	0	10	0	1	0	4
277		6	max	59.92	2	.922	6	.111	1	0	4	0	3	0	15
278			min	-44.332	9	.215	15	-.599	5	0	10	0	1	0	4
279		7	max	59.853	2	.744	6	.111	1	0	4	0	3	0	15
280			min	-44.388	9	.173	15	-.466	5	0	10	0	4	0	4
281		8	max	59.786	2	.566	6	.111	1	0	4	0	3	0	15
282			min	-44.444	9	.131	15	-.332	5	0	10	0	4	0	4
283		9	max	59.719	2	.388	6	.111	1	0	4	0	3	0	15
284			min	-44.5	9	.089	15	-.199	5	0	10	0	4	-.001	4
285		10	max	59.652	2	.21	6	.111	1	0	4	0	3	0	15
286			min	-44.556	9	.047	15	-.065	5	0	10	0	4	-.001	4
287		11	max	59.585	2	.041	2	.111	1	0	4	0	3	0	15
288			min	-44.612	9	.005	12	-.029	3	0	10	0	4	-.001	4
289		12	max	59.517	2	-.036	15	.227	4	0	4	0	3	0	15
290			min	-44.668	9	-.146	4	-.029	3	0	10	0	4	-.001	4
291		13	max	59.45	2	-.078	15	.361	4	0	4	0	3	0	15
292			min	-44.723	9	-.324	4	-.029	3	0	10	0	5	-.001	4
293		14	max	59.383	2	-.12	15	.494	4	0	4	0	3	0	15
294			min	-44.779	9	-.502	4	-.029	3	0	10	0	5	-.001	4
295		15	max	59.316	2	-.162	15	.628	4	0	4	0	3	0	15
296			min	-44.835	9	-.68	4	-.029	3	0	10	0	5	0	4
297		16	max	59.249	2	-.204	15	.761	4	0	4	0	3	0	15
298			min	-44.891	9	-.858	4	-.029	3	0	10	0	10	0	4
299		17	max	59.182	2	-.245	15	.895	4	0	4	0	3	0	15
300			min	-44.947	9	-1.036	4	-.029	3	0	10	0	10	0	4
301		18	max	59.115	2	-.287	15	1.028	4	0	4	0	4	0	15
302			min	-45.003	9	-1.214	4	-.029	3	0	10	0	10	0	4
303		19	max	59.048	2	-.329	15	1.162	4	0	4	0	4	0	1
304			min	-45.059	9	-1.392	4	-.029	3	0	10	0	10	0	1
305	M12	1	max	338.525	1	0	1	.357	1	0	1	0	4	0	1
306			min	-120.074	3	0	1	-12.836	5	0	1	0	3	0	1
307		2	max	338.589	1	0	1	.357	1	0	1	0	1	0	1
308			min	-120.026	3	0	1	-12.892	5	0	1	-.001	5	0	1
309		3	max	338.654	1	0	1	.357	1	0	1	0	1	0	1
310			min	-119.977	3	0	1	-12.948	5	0	1	-.002	5	0	1
311		4	max	338.719	1	0	1	.357	1	0	1	0	1	0	1
312			min	-119.929	3	0	1	-13.004	5	0	1	-.003	5	0	1
313		5	max	338.783	1	0	1	.357	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-119.88	3	0	1	-13.06	5	0	1	-.005	5	0	1
315		6	max	338.848	1	0	1	.357	1	0	1	0	1	0	1
316			min	-119.832	3	0	1	-13.116	5	0	1	-.006	5	0	1
317		7	max	338.913	1	0	1	.357	1	0	1	0	1	0	1
318			min	-119.783	3	0	1	-13.172	5	0	1	-.007	5	0	1
319		8	max	338.978	1	0	1	.357	1	0	1	0	1	0	1
320			min	-119.735	3	0	1	-13.228	5	0	1	-.008	5	0	1
321		9	max	339.042	1	0	1	.357	1	0	1	0	1	0	1
322			min	-119.686	3	0	1	-13.285	5	0	1	-.009	5	0	1
323		10	max	339.107	1	0	1	.357	1	0	1	0	1	0	1
324			min	-119.637	3	0	1	-13.341	5	0	1	-.011	5	0	1
325		11	max	339.172	1	0	1	.357	1	0	1	0	1	0	1
326			min	-119.589	3	0	1	-13.397	5	0	1	-.012	5	0	1
327		12	max	339.236	1	0	1	.357	1	0	1	0	1	0	1
328			min	-119.54	3	0	1	-13.453	5	0	1	-.013	5	0	1
329		13	max	339.301	1	0	1	.357	1	0	1	0	1	0	1
330			min	-119.492	3	0	1	-13.509	5	0	1	-.014	5	0	1
331		14	max	339.366	1	0	1	.357	1	0	1	0	1	0	1
332			min	-119.443	3	0	1	-13.565	5	0	1	-.015	5	0	1
333		15	max	339.431	1	0	1	.357	1	0	1	0	1	0	1
334			min	-119.395	3	0	1	-13.621	5	0	1	-.017	5	0	1
335		16	max	339.495	1	0	1	.357	1	0	1	0	1	0	1
336			min	-119.346	3	0	1	-13.677	5	0	1	-.018	5	0	1
337		17	max	339.56	1	0	1	.357	1	0	1	0	1	0	1
338			min	-119.298	3	0	1	-13.733	5	0	1	-.019	5	0	1
339		18	max	339.625	1	0	1	.357	1	0	1	0	1	0	1
340			min	-119.249	3	0	1	-13.789	5	0	1	-.02	5	0	1
341		19	max	339.689	1	0	1	.357	1	0	1	0	1	0	1
342			min	-119.201	3	0	1	-13.845	5	0	1	-.021	5	0	1
343	M1	1	max	47.91	1	347.945	3	.652	10	0	1	.026	4	.016	2
344			min	2.326	10	-248.727	1	-14.614	4	0	3	-.001	10	-.018	3
345		2	max	47.982	1	347.743	3	.652	10	0	1	.023	4	.069	1
346			min	2.386	10	-248.997	1	-14.372	4	0	3	-.001	10	-.094	3
347		3	max	57.1	1	4.481	4	.649	10	0	5	.019	4	.122	1
348			min	-5.572	3	-23.482	3	-13.323	4	0	1	0	10	-.167	3
349		4	max	57.172	1	4.134	4	.649	10	0	5	.016	4	.123	1
350			min	-5.517	3	-23.684	3	-13.081	4	0	1	0	10	-.162	3
351		5	max	57.244	1	3.788	4	.649	10	0	5	.014	4	.125	2
352			min	-5.463	3	-23.887	3	-12.839	4	0	1	0	10	-.157	3
353		6	max	57.317	1	3.5	14	.649	10	0	5	.011	4	.129	2
354			min	-5.409	3	-24.089	3	-12.597	4	0	1	0	10	-.152	3
355		7	max	57.389	1	3.235	14	.649	10	0	5	.008	4	.133	2
356			min	-5.355	3	-24.291	3	-12.355	4	0	1	0	10	-.147	3
357		8	max	57.461	1	2.97	14	.649	10	0	5	.005	4	.137	2
358			min	-5.301	3	-24.494	3	-12.113	4	0	1	0	10	-.141	3
359		9	max	57.533	1	2.705	14	.649	10	0	5	.003	4	.141	2
360			min	-5.246	3	-24.696	3	-11.871	4	0	1	0	10	-.136	3
361		10	max	57.606	1	2.44	14	.649	10	0	5	.001	3	.145	2
362			min	-5.192	3	-24.898	3	-11.629	4	0	1	0	10	-.131	3
363		11	max	57.678	1	2.175	14	.649	10	0	5	0	3	.149	2
364			min	-5.138	3	-25.1	3	-11.387	4	0	1	-.002	4	-.125	3
365		12	max	57.75	1	1.91	14	.649	10	0	5	0	10	.153	2
366			min	-5.084	3	-25.303	3	-11.145	4	0	1	-.005	4	-.12	3
367		13	max	57.823	1	1.645	14	.649	10	0	5	0	10	.157	2
368			min	-5.03	3	-25.505	3	-10.903	4	0	1	-.007	4	-.114	3
369		14	max	57.895	1	1.412	9	.649	10	0	5	0	10	.162	2
370			min	-4.975	3	-25.707	3	-10.661	4	0	1	-.009	4	-.109	3



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 Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	57.967	1	1.188	9	.649	10	0	5	0	10	.166	2
372		min	-4.921	3	-25.91	3	-10.419	4	0	1	-.012	4	-.103	3
373	16	max	70.915	2	17.956	2	.656	10	0	1	0	10	.17	2
374		min	-34.722	3	-52.863	3	-9.77	1	0	4	-.014	4	-.097	3
375	17	max	70.988	2	17.686	2	.656	10	0	1	.001	10	.166	2
376		min	-34.668	3	-53.065	3	-9.77	1	0	4	-.016	4	-.086	3
377	18	max	-2.267	12	341.259	2	.685	10	0	5	.001	10	.093	2
378		min	-47.914	1	-169.712	3	-22.065	4	0	2	-.021	4	-.049	3
379	19	max	-2.23	12	340.989	2	.685	10	0	5	.001	10	.019	2
380		min	-47.841	1	-169.914	3	-21.823	4	0	2	-.025	4	-.013	3
381	M5	1	max	122.793	1	1079.1	3	0	1	0	.03	4	.036	3
382		min	-3.483	3	-763.803	1	-58.847	3	0	3	0	11	-.031	2
383	2	max	122.866	1	1078.897	3	0	1	0	9	.026	4	.135	1
384		min	-3.429	3	-764.072	1	-58.847	3	0	3	-.003	3	-.198	3
385	3	max	149.632	1	6.036	9	6.041	3	0	3	.022	4	.298	1
386		min	-43.327	3	-70.904	3	-16.656	4	0	4	-.015	3	-.427	3
387	4	max	149.705	1	5.812	9	6.041	3	0	3	.018	4	.302	1
388		min	-43.273	3	-71.106	3	-16.414	4	0	4	-.014	3	-.411	3
389	5	max	149.777	1	5.587	9	6.041	3	0	3	.014	4	.306	1
390		min	-43.219	3	-71.309	3	-16.172	4	0	4	-.012	3	-.396	3
391	6	max	149.849	1	5.362	9	6.041	3	0	3	.011	4	.314	2
392		min	-43.165	3	-71.511	3	-15.93	4	0	4	-.011	3	-.38	3
393	7	max	149.921	1	5.137	9	6.041	3	0	3	.007	4	.325	2
394		min	-43.11	3	-71.713	3	-15.688	4	0	4	-.01	3	-.365	3
395	8	max	149.994	1	4.913	9	6.041	3	0	3	.004	4	.336	2
396		min	-43.056	3	-71.915	3	-15.446	4	0	4	-.009	3	-.349	3
397	9	max	150.066	1	4.688	9	6.041	3	0	3	0	4	.348	2
398		min	-43.002	3	-72.118	3	-15.204	4	0	4	-.007	3	-.334	3
399	10	max	150.138	1	4.463	9	6.041	3	0	3	0	1	.359	2
400		min	-42.948	3	-72.32	3	-14.962	4	0	4	-.006	3	-.318	3
401	11	max	150.21	1	4.238	9	6.041	3	0	3	0	1	.37	2
402		min	-42.894	3	-72.522	3	-14.72	4	0	4	-.006	4	-.302	3
403	12	max	150.283	1	4.014	9	6.041	3	0	3	0	1	.382	2
404		min	-42.839	3	-72.725	3	-14.478	4	0	4	-.009	4	-.287	3
405	13	max	150.355	1	3.789	9	6.041	3	0	3	0	1	.393	2
406		min	-42.785	3	-72.927	3	-14.236	4	0	4	-.012	4	-.271	3
407	14	max	150.427	1	3.564	9	6.041	3	0	3	0	1	.405	2
408		min	-42.731	3	-73.129	3	-13.994	4	0	4	-.015	4	-.255	3
409	15	max	150.5	1	3.339	9	6.041	3	0	3	0	3	.416	2
410		min	-42.677	3	-73.332	3	-13.752	4	0	4	-.018	4	-.239	3
411	16	max	213.753	2	64.79	2	6.027	3	0	3	.001	3	.427	2
412		min	-104.506	3	-127.305	3	-12.526	4	0	4	-.021	4	-.223	3
413	17	max	213.825	2	64.52	2	6.027	3	0	3	.003	3	.413	2
414		min	-104.452	3	-127.507	3	-12.284	4	0	4	-.024	4	-.195	3
415	18	max	-1.509	12	1050.201	2	5.602	3	0	4	.004	3	.189	2
416		min	-123.002	1	-511.358	3	-25.715	5	0	9	-.029	4	-.086	3
417	19	max	-1.472	12	1049.931	2	5.602	3	0	4	.005	3	.025	3
418		min	-122.93	1	-511.561	3	-25.473	5	0	9	-.035	4	-.039	2
419	M9	1	max	47.909	1	347.893	3	107.869	4	0	.001	10	.016	2
420		min	-.572	5	-248.727	1	-.652	10	0	1	-.019	1	-.018	3
421	2	max	47.982	1	347.691	3	108.111	4	0	3	.023	5	.069	1
422		min	-.539	5	-248.997	1	-.652	10	0	1	-.017	1	-.094	3
423	3	max	57.456	1	3.872	9	9.665	1	0	4	.044	5	.122	1
424		min	-5.932	3	-23.403	3	-21.402	5	0	10	-.015	1	-.167	3
425	4	max	57.529	1	3.647	9	9.665	1	0	4	.04	5	.123	1
426		min	-5.878	3	-23.605	3	-21.16	5	0	10	-.013	1	-.162	3
427	5	max	57.601	1	3.422	9	9.665	1	0	4	.035	5	.125	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-5.824	3	-23.807	3	-20.918	5	0	10	-.011	1	-.157	3
429		6	max	57.673	1	3.198	9	9.665	1	0	4	.03	5	.129	2
430			min	-5.77	3	-24.01	3	-20.676	5	0	10	-.009	1	-.152	3
431		7	max	57.745	1	2.973	9	9.665	1	0	4	.026	5	.133	2
432			min	-5.715	3	-24.212	3	-20.434	5	0	10	-.006	1	-.147	3
433		8	max	57.818	1	2.748	9	9.665	1	0	4	.022	5	.137	2
434			min	-5.661	3	-24.414	3	-20.192	5	0	10	-.004	1	-.141	3
435		9	max	57.89	1	2.523	9	9.665	1	0	4	.017	5	.141	2
436			min	-5.607	3	-24.617	3	-19.95	5	0	10	-.002	1	-.136	3
437		10	max	57.962	1	2.298	9	9.665	1	0	4	.013	4	.145	2
438			min	-5.553	3	-24.819	3	-19.708	5	0	10	0	1	-.131	3
439		11	max	58.035	1	2.074	9	9.665	1	0	4	.009	4	.149	2
440			min	-5.499	3	-25.021	3	-19.466	5	0	10	0	10	-.125	3
441		12	max	58.107	1	1.849	9	9.665	1	0	4	.006	3	.153	2
442			min	-5.444	3	-25.224	3	-19.224	5	0	10	0	10	-.12	3
443		13	max	58.179	1	1.624	9	9.665	1	0	4	.006	1	.157	2
444			min	-5.39	3	-25.426	3	-18.982	5	0	10	0	10	-.114	3
445		14	max	58.251	1	1.399	9	9.665	1	0	4	.008	1	.161	2
446			min	-5.336	3	-25.628	3	-18.74	5	0	10	-.004	5	-.109	3
447		15	max	58.324	1	1.175	9	9.665	1	0	4	.01	1	.166	2
448			min	-5.282	3	-25.83	3	-18.498	5	0	10	-.008	5	-.103	3
449		16	max	70.979	2	17.721	2	9.769	1	0	10	.013	1	.17	2
450			min	-35.577	3	-53.225	3	-17.121	5	0	4	-.011	5	-.097	3
451		17	max	71.051	2	17.451	2	9.769	1	0	10	.015	1	.166	2
452			min	-35.523	3	-53.428	3	-16.879	5	0	4	-.015	5	-.086	3
453		18	max	10.377	5	341.259	2	10.121	1	0	2	.017	1	.093	2
454			min	-47.913	1	-169.705	3	-29.467	5	0	3	-.021	5	-.049	3
455		19	max	10.411	5	340.989	2	10.121	1	0	2	.019	1	.019	2
456			min	-47.841	1	-169.908	3	-29.225	5	0	3	-.027	5	-.013	3
457	M13	1	max	107.868	4	248.565	1	.572	5	.016	2	.019	1	0	1
458			min	-.652	10	-347.918	3	-47.908	1	-.018	3	-.001	10	0	3
459		2	max	103.742	4	178.573	1	1.147	5	.016	2	.011	3	.124	3
460			min	-.652	10	-249.311	3	-35.763	1	-.018	3	-.002	2	-.089	1
461		3	max	99.617	4	108.581	1	1.722	5	.016	2	.009	3	.208	3
462			min	-.652	10	-150.705	3	-23.618	1	-.018	3	-.011	1	-.149	1
463		4	max	95.491	4	38.589	1	2.297	5	.016	2	.007	3	.25	3
464			min	-.652	10	-52.098	3	-11.473	1	-.018	3	-.018	1	-.179	1
465		5	max	91.365	4	46.508	3	2.872	5	.016	2	.005	3	.251	3
466			min	-.652	10	-31.403	1	-4.108	3	-.018	3	-.02	1	-.181	1
467		6	max	87.24	4	145.115	3	12.818	1	.016	2	.004	5	.211	3
468			min	-.652	10	-101.395	1	-3.564	3	-.018	3	-.017	1	-.153	1
469		7	max	83.114	4	243.721	3	24.963	1	.016	2	.006	5	.13	3
470			min	-.652	10	-171.387	1	-3.021	3	-.018	3	-.009	1	-.096	1
471		8	max	78.988	4	342.328	3	37.108	1	.016	2	.008	4	.008	3
472			min	-.652	10	-241.379	1	-2.477	3	-.018	3	0	12	-.012	2
473		9	max	74.863	4	440.935	3	49.253	1	.016	2	.021	1	.105	1
474			min	-.652	10	-311.371	1	-1.933	3	-.018	3	0	3	-.155	3
475		10	max	70.737	4	11.834	5	61.398	1	.016	2	.044	1	.249	1
476			min	-.652	10	-539.541	3	1.205	12	-.018	3	-.018	5	-.359	3
477		11	max	47.606	4	311.371	1	8.489	5	.018	3	.021	1	.105	1
478			min	-.652	10	-440.935	3	-49.253	1	-.016	2	-.014	5	-.155	3
479		12	max	43.48	4	241.379	1	9.064	5	.018	3	.004	2	.008	3
480			min	-.652	10	-342.328	3	-37.108	1	-.016	2	-.011	5	-.012	2
481		13	max	39.354	4	171.387	1	9.639	5	.018	3	0	10	.13	3
482			min	-.652	10	-243.721	3	-24.963	1	-.016	2	-.009	1	-.096	1
483		14	max	35.229	4	101.395	1	10.214	5	.018	3	0	10	.211	3
484			min	-.652	10	-145.115	3	-12.817	1	-.016	2	-.017	1	-.153	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	31.103	4	31.403	1	11.021	4	.018	3	.002	5	.251	3
486			min	-.652	10	-46.508	3	-2.655	2	-.016	2	-.02	1	-.181	1
487		16	max	26.977	4	52.098	3	13.966	4	.018	3	.006	5	.25	3
488			min	-.652	10	-38.589	1	-.754	10	-.016	2	-.018	1	-.179	1
489		17	max	22.852	4	150.705	3	23.618	1	.018	3	.011	5	.208	3
490			min	-.652	10	-108.581	1	.273	10	-.016	2	-.011	1	-.149	1
491		18	max	18.726	4	249.312	3	35.763	1	.018	3	.017	4	.124	3
492			min	-.652	10	-178.573	1	1.299	10	-.016	2	-.002	2	-.089	1
493		19	max	14.6	4	347.918	3	47.908	1	.018	3	.026	4	0	1
494			min	-.652	10	-248.565	1	2.326	10	-.016	2	-.001	10	0	3
495	M16	1	max	29.214	5	341.057	2	10.411	5	.013	3	.019	1	0	2
496			min	-10.11	1	-169.923	3	-47.843	1	-.019	2	-.027	5	0	3
497		2	max	25.088	5	244.801	2	10.986	5	.013	3	.002	9	.061	3
498			min	-10.11	1	-122.622	3	-35.698	1	-.019	2	-.023	5	-.122	2
499		3	max	20.962	5	148.545	2	11.561	5	.013	3	0	12	.102	3
500			min	-10.11	1	-75.321	3	-23.552	1	-.019	2	-.02	4	-.204	2
501		4	max	16.837	5	52.288	2	12.136	5	.013	3	-.001	12	.124	3
502			min	-10.11	1	-28.02	3	-11.407	1	-.019	2	-.018	1	-.246	2
503		5	max	12.711	5	19.282	3	12.711	5	.013	3	-.002	10	.126	3
504			min	-10.11	1	-43.968	2	-2.196	3	-.019	2	-.02	1	-.248	2
505		6	max	8.585	5	66.583	3	15.569	4	.013	3	0	10	.108	3
506			min	-10.11	1	-140.224	2	-1.652	3	-.019	2	-.017	1	-.209	2
507		7	max	4.46	5	113.884	3	25.028	1	.013	3	.003	5	.07	3
508			min	-10.11	1	-236.481	2	-1.108	3	-.019	2	-.009	1	-.131	2
509		8	max	2.767	3	161.185	3	37.173	1	.013	3	.009	4	.013	3
510			min	-10.11	1	-332.737	2	-.564	3	-.019	2	-.005	3	-.012	2
511		9	max	2.767	3	208.487	3	49.318	1	.013	3	.022	1	.147	2
512			min	-10.11	1	-428.993	2	-.02	3	-.019	2	-.005	3	-.064	3
513		10	max	17.559	5	-7.534	15	61.464	1	.005	14	.045	1	.345	2
514			min	-10.11	1	-525.25	2	-1.479	3	-.019	2	-.005	3	-.161	3
515		11	max	13.434	5	428.993	2	7.143	5	.019	2	.022	1	.147	2
516			min	-10.11	1	-208.487	3	-49.318	1	-.013	3	-.01	5	-.064	3
517		12	max	9.308	5	332.737	2	7.718	5	.019	2	.004	2	.013	3
518			min	-10.11	1	-161.185	3	-37.173	1	-.013	3	-.007	5	-.012	2
519		13	max	5.182	5	236.481	2	8.293	5	.019	2	0	10	.07	3
520			min	-10.11	1	-113.884	3	-25.028	1	-.013	3	-.009	1	-.131	2
521		14	max	1.057	5	140.224	2	8.867	5	.019	2	0	12	.108	3
522			min	-10.11	1	-66.583	3	-12.883	1	-.013	3	-.017	1	-.209	2
523		15	max	.685	10	43.968	2	9.647	4	.019	2	.003	5	.126	3
524			min	-10.11	1	-19.282	3	-2.713	2	-.013	3	-.02	1	-.248	2
525		16	max	.685	10	28.02	3	12.592	4	.019	2	.007	5	.124	3
526			min	-10.11	1	-52.288	2	-.783	10	-.013	3	-.018	1	-.246	2
527		17	max	.685	10	75.321	3	23.553	1	.019	2	.012	5	.102	3
528			min	-13.586	4	-148.545	2	.243	10	-.013	3	-.011	1	-.204	2
529		18	max	.685	10	122.622	3	35.698	1	.019	2	.017	4	.061	3
530			min	-17.711	4	-244.801	2	1.27	10	-.013	3	-.002	2	-.122	2
531		19	max	.685	10	169.923	3	47.843	1	.019	2	.025	4	0	2
532			min	-21.837	4	-341.057	2	2.23	12	-.013	3	-.001	10	0	5
533	M15	1	max	0	1	.774	3	.141	3	0	1	0	1	0	1
534			min	-78.093	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.688	3	.141	3	0	1	0	1	0	1
536			min	-78.147	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.602	3	.141	3	0	1	0	1	0	1
538			min	-78.201	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.516	3	.141	3	0	1	0	1	0	1
540			min	-78.255	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.43	3	.141	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-78.309	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.344	3	.141	3	0	1	0	1	0	1
544		min	-78.363	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.258	3	.141	3	0	1	0	3	0	1
546		min	-78.417	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.172	3	.141	3	0	1	0	3	0	1
548		min	-78.471	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.086	3	.141	3	0	1	0	3	0	1
550		min	-78.525	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.141	3	0	1	0	3	0	1
552		min	-78.579	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.141	3	0	1	0	3	0	1
554		min	-78.633	3	-.086	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.141	3	0	1	0	3	0	1
556		min	-78.687	3	-.172	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.141	3	0	1	0	3	0	1
558		min	-78.741	3	-.258	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.141	3	0	1	0	3	0	1
560		min	-78.795	3	-.344	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.141	3	0	1	0	3	0	1
562		min	-78.849	3	-.43	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.141	3	0	1	0	3	0	1
564		min	-78.903	3	-.516	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.141	3	0	1	0	3	0	1
566		min	-78.957	3	-.602	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.141	3	0	1	0	3	0	1
568		min	-79.011	3	-.688	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.141	3	0	1	0	3	0	1
570		min	-79.065	3	-.774	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.875	.242	4	0	3	0	3	0	1
572		min	-155.988	4	0	1	-.066	3	0	14	0	4	0	1
573	2	max	0	1	1.667	4	.219	4	0	3	0	3	0	1
574		min	-155.993	4	0	1	-.066	3	0	14	0	4	0	4
575	3	max	0	1	1.458	4	.197	4	0	3	0	3	0	1
576		min	-155.998	4	0	1	-.066	3	0	14	0	4	0	4
577	4	max	0	1	1.25	4	.174	4	0	3	0	3	0	1
578		min	-156.003	4	0	1	-.066	3	0	14	0	4	-.001	4
579	5	max	0	1	1.042	4	.152	4	0	3	0	3	0	1
580		min	-156.008	4	0	1	-.066	3	0	14	0	9	-.001	4
581	6	max	0	1	.833	4	.129	4	0	3	0	3	0	1
582		min	-156.013	4	0	1	-.066	3	0	14	0	9	-.002	4
583	7	max	0	1	.625	4	.107	4	0	3	0	3	0	1
584		min	-156.018	4	0	1	-.066	3	0	14	0	9	-.002	4
585	8	max	0	1	.417	4	.084	4	0	3	0	3	0	1
586		min	-156.024	4	0	1	-.066	3	0	14	0	9	-.002	4
587	9	max	0	1	.208	4	.062	4	0	3	0	5	0	1
588		min	-156.029	4	0	1	-.066	3	0	14	0	9	-.002	4
589	10	max	0	1	0	1	.039	4	0	3	0	5	0	1
590		min	-156.034	4	0	1	-.066	3	0	14	0	9	-.002	4
591	11	max	0	1	0	1	.022	14	0	3	0	5	0	1
592		min	-156.039	4	-.208	4	-.066	3	0	14	0	9	-.002	4
593	12	max	0	1	0	1	.021	9	0	3	0	5	0	1
594		min	-156.044	4	-.417	4	-.066	3	0	14	0	9	-.002	4
595	13	max	0	1	0	1	.021	9	0	3	0	5	0	1
596		min	-156.049	4	-.625	4	-.066	3	0	14	0	9	-.002	4
597	14	max	0	1	0	1	.021	9	0	3	0	5	0	1
598		min	-156.055	4	-.833	4	-.066	3	0	14	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.031	9	0	1	.021	9	0	3	0	4	0	1
600		min	-156.06	4	-1.042	4	-.078	5	0	14	0	3	-.001	4
601	16	max	.091	9	0	1	.021	9	0	3	0	4	0	1
602		min	-156.065	4	-1.25	4	-.101	5	0	14	0	3	-.001	4
603	17	max	.151	9	0	1	.021	9	0	3	0	14	0	1
604		min	-156.107	5	-1.458	4	-.123	5	0	14	0	3	0	4
605	18	max	.211	9	0	1	.021	9	0	3	0	9	0	1
606		min	-156.171	5	-1.667	4	-.146	5	0	14	0	3	0	4
607	19	max	.271	9	0	1	.021	9	0	3	0	9	0	1
608		min	-156.235	5	-1.875	4	-.169	5	0	14	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.006	2	.001	1	7.486e-4	5	NC	3	NC	1	
2			min	-.003	3	-.005	3	-.008	5	-1.459e-4	1	4764.458	2	NC	1	
3			2	max	.002	1	.006	2	.001	1	7.681e-4	5	NC	3	NC	1
4				min	-.003	3	-.005	3	-.007	5	-1.393e-4	1	5159.125	2	NC	1
5			3	max	.002	1	.005	2	.001	1	7.876e-4	5	NC	3	NC	1
6				min	-.002	3	-.005	3	-.007	5	-1.328e-4	1	5621.764	2	NC	1
7			4	max	.002	1	.005	2	0	1	8.072e-4	5	NC	3	NC	1
8				min	-.002	3	-.005	3	-.007	5	-1.263e-4	1	6167.765	2	NC	1
9			5	max	.001	1	.004	2	0	1	8.267e-4	5	NC	1	NC	1
10				min	-.002	3	-.004	3	-.007	5	-1.198e-4	1	6817.356	2	NC	1
11			6	max	.001	1	.004	2	0	1	8.462e-4	5	NC	1	NC	1
12				min	-.002	3	-.004	3	-.006	5	-1.133e-4	1	7597.56	2	NC	1
13			7	max	.001	1	.004	2	0	1	8.658e-4	5	NC	1	NC	1
14				min	-.002	3	-.004	3	-.006	5	-1.067e-4	1	8545.157	2	NC	1
15			8	max	.001	1	.003	2	0	1	8.853e-4	5	NC	1	NC	1
16				min	-.002	3	-.004	3	-.005	5	-1.002e-4	1	9711.317	2	NC	1
17			9	max	.001	1	.003	2	0	1	9.048e-4	5	NC	1	NC	1
18				min	-.002	3	-.003	3	-.005	5	-9.371e-5	1	NC	1	NC	1
19			10	max	0	1	.002	2	0	1	9.244e-4	5	NC	1	NC	1
20				min	-.001	3	-.003	3	-.005	5	-8.719e-5	1	NC	1	NC	1
21		11	max	0	1	.002	2	0	1	9.439e-4	5	NC	1	NC	1	
22			min	-.001	3	-.003	3	-.004	5	-8.066e-5	1	NC	1	NC	1	
23		12	max	0	1	.002	2	0	1	9.634e-4	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.004	5	-7.414e-5	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	9.83e-4	5	NC	1	NC	1	
26			min	0	3	-.002	3	-.003	5	-6.762e-5	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.002e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.003	5	-6.11e-5	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.022e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.002	5	-5.458e-5	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.042e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.002	5	-4.806e-5	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.061e-3	5	NC	1	NC	1	
34			min	0	3	0	3	-.001	5	-4.154e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.081e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-3.502e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.1e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.85e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.304e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.007e-4	5	NC	1	NC	1	
41		2	max	0	14	0	2	.003	5	1.918e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-5.028e-4	5	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	14	0	2	.005	5	2.532e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	9	-5.05e-4	5	NC	1	NC	1
45		4	max	0	14	0	2	.008	4	3.145e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	9	-5.071e-4	5	NC	1	NC	1
47		5	max	0	14	0	2	.011	4	3.759e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.093e-4	5	NC	1	NC	1
49		6	max	0	14	0	2	.013	4	4.373e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.114e-4	5	NC	1	NC	1
51		7	max	0	14	0	2	.016	4	4.987e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-5.135e-4	5	NC	1	NC	1
53		8	max	0	14	.001	2	.019	4	5.601e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.157e-4	5	NC	1	NC	1
55		9	max	0	14	.001	2	.021	4	6.214e-5	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-5.178e-4	5	NC	1	NC	1
57		10	max	0	14	.002	2	.024	4	6.828e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.2e-4	5	NC	1	NC	1
59		11	max	0	14	.002	2	.026	4	7.442e-5	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-5.221e-4	5	NC	1	NC	1
61		12	max	0	14	.003	2	.028	4	8.056e-5	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-5.243e-4	5	NC	1	NC	1
63		13	max	0	14	.004	2	.03	4	8.67e-5	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-5.264e-4	5	NC	1	NC	1
65		14	max	0	14	.004	2	.032	4	9.283e-5	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-5.285e-4	5	NC	1	NC	1
67		15	max	0	14	.005	2	.034	4	9.897e-5	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-5.307e-4	5	8821.207	2	NC	1
69		16	max	0	14	.006	2	.036	4	1.051e-4	1	NC	1	NC	1
70			min	0	2	-.008	3	0	10	-5.328e-4	5	7518.467	2	NC	1
71		17	max	0	14	.007	2	.038	4	1.112e-4	1	NC	3	NC	1
72			min	0	2	-.008	3	0	10	-5.35e-4	5	6499.597	2	NC	1
73		18	max	0	14	.008	2	.04	4	1.174e-4	1	NC	3	NC	1
74			min	0	2	-.008	3	0	10	-5.371e-4	5	5695.615	2	NC	1
75		19	max	0	14	.009	2	.042	4	1.235e-4	1	NC	3	NC	1
76			min	0	2	-.008	3	0	10	-5.393e-4	5	5056.749	2	NC	1
77	M4	1	max	.002	1	.007	2	0	10	1.734e-3	5	NC	1	NC	1
78			min	0	3	-.006	3	-.044	4	-1.2e-4	1	NC	1	438.399	4
79		2	max	.002	1	.007	2	0	10	1.734e-3	5	NC	1	NC	1
80			min	0	3	-.005	3	-.04	4	-1.2e-4	1	NC	1	477.845	4
81		3	max	.001	1	.006	2	0	10	1.734e-3	5	NC	1	NC	1
82			min	0	3	-.005	3	-.037	4	-1.2e-4	1	NC	1	524.785	4
83		4	max	.001	1	.006	2	0	10	1.734e-3	5	NC	1	NC	1
84			min	0	3	-.005	3	-.033	4	-1.2e-4	1	NC	1	581.192	4
85		5	max	.001	1	.006	2	0	10	1.734e-3	5	NC	1	NC	1
86			min	0	3	-.004	3	-.03	4	-1.2e-4	1	NC	1	649.754	4
87		6	max	.001	1	.005	2	0	10	1.734e-3	5	NC	1	NC	1
88			min	0	3	-.004	3	-.026	4	-1.2e-4	1	NC	1	734.209	4
89		7	max	.001	1	.005	2	0	10	1.734e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.023	4	-1.2e-4	1	NC	1	839.881	4
91		8	max	0	1	.004	2	0	10	1.734e-3	5	NC	1	NC	1
92			min	0	3	-.003	3	-.02	4	-1.2e-4	1	NC	1	974.563	4
93		9	max	0	1	.004	2	0	10	1.734e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.017	4	-1.2e-4	1	NC	1	1150.043	4
95		10	max	0	1	.004	2	0	10	1.734e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.014	4	-1.2e-4	1	NC	1	1384.849	4
97		11	max	0	1	.003	2	0	10	1.734e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.011	4	-1.2e-4	1	NC	1	1709.495	4
99		12	max	0	1	.003	2	0	10	1.734e-3	5	NC	1	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.002	3	-.009	4	-1.2e-4	1	NC	1	2177.2	4
101		13	max	0	1	.002	2	0	10	1.734e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.007	4	-1.2e-4	1	NC	1	2887.785	4
103		14	max	0	1	.002	2	0	10	1.734e-3	5	NC	1	NC	1
104			min	0	3	-.002	3	-.005	4	-1.2e-4	1	NC	1	4047.132	4
105		15	max	0	1	.002	2	0	10	1.734e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.003	4	-1.2e-4	1	NC	1	6138.89	4
107		16	max	0	1	.001	2	0	10	1.734e-3	5	NC	1	NC	1
108			min	0	3	0	3	-.002	4	-1.2e-4	1	NC	1	NC	1
109		17	max	0	1	0	2	0	10	1.734e-3	5	NC	1	NC	1
110			min	0	3	0	3	0	4	-1.2e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	1.734e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-1.2e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	1.734e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.2e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.015	2	0	9	7.911e-4	4	NC	3	NC	1
116			min	-.008	3	-.012	3	-.008	5	-9.223e-8	1	1956.62	2	7284.908	3
117		2	max	.005	1	.014	2	0	9	8.107e-4	4	NC	3	NC	1
118			min	-.008	3	-.011	3	-.007	5	-8.731e-8	1	2091.9	2	7799.947	3
119		3	max	.005	1	.013	2	0	9	8.304e-4	4	NC	3	NC	1
120			min	-.007	3	-.011	3	-.007	5	-8.239e-8	1	2246.722	2	8403.374	3
121		4	max	.005	1	.012	2	0	9	8.5e-4	4	NC	3	NC	1
122			min	-.007	3	-.01	3	-.007	5	-7.747e-8	1	2425.032	2	9114.949	3
123		5	max	.004	1	.011	2	0	9	8.697e-4	4	NC	3	NC	1
124			min	-.006	3	-.009	3	-.006	5	-9.859e-7	9	2631.919	2	9960.698	3
125		6	max	.004	1	.01	2	0	9	8.893e-4	4	NC	3	NC	1
126			min	-.006	3	-.009	3	-.006	5	-1.908e-6	9	2874.048	2	NC	1
127		7	max	.004	1	.01	2	0	9	9.09e-4	4	NC	3	NC	1
128			min	-.005	3	-.008	3	-.006	5	-2.83e-6	9	3160.322	2	NC	1
129		8	max	.003	1	.009	2	0	9	9.286e-4	4	NC	3	NC	1
130			min	-.005	3	-.008	3	-.005	5	-3.752e-6	9	3502.899	2	NC	1
131		9	max	.003	1	.008	2	0	9	9.483e-4	4	NC	3	NC	1
132			min	-.005	3	-.007	3	-.005	5	-4.673e-6	9	3918.813	2	NC	1
133		10	max	.003	1	.007	2	0	9	9.68e-4	4	NC	3	NC	1
134			min	-.004	3	-.006	3	-.005	5	-5.595e-6	9	4432.687	2	NC	1
135		11	max	.002	1	.006	2	0	9	9.876e-4	4	NC	3	NC	1
136			min	-.004	3	-.006	3	-.004	5	-6.517e-6	9	5081.479	2	NC	1
137		12	max	.002	1	.005	2	0	9	1.007e-3	4	NC	3	NC	1
138			min	-.003	3	-.005	3	-.004	5	-7.439e-6	9	5923.283	2	NC	1
139		13	max	.002	1	.004	2	0	9	1.027e-3	4	NC	1	NC	1
140			min	-.003	3	-.004	3	-.003	5	-8.361e-6	9	7054.951	2	NC	1
141		14	max	.002	1	.003	2	0	9	1.047e-3	4	NC	1	NC	1
142			min	-.002	3	-.004	3	-.003	5	-9.283e-6	9	8650.842	2	NC	1
143		15	max	.001	1	.003	2	0	9	1.066e-3	4	NC	1	NC	1
144			min	-.002	3	-.003	3	-.002	5	-1.02e-5	9	NC	1	NC	1
145		16	max	0	1	.002	2	0	9	1.086e-3	4	NC	1	NC	1
146			min	-.001	3	-.002	3	-.002	5	-1.113e-5	9	NC	1	NC	1
147		17	max	0	1	.001	2	0	9	1.106e-3	4	NC	1	NC	1
148			min	0	3	-.002	3	-.001	5	-1.205e-5	9	NC	1	NC	1
149		18	max	0	1	0	2	0	9	1.125e-3	4	NC	1	NC	1
150			min	0	3	0	3	0	4	-1.297e-5	9	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.145e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.389e-5	9	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	6.301e-6	9	NC	1	NC	1
154			min	0	1	0	1	0	1	-5.21e-4	4	NC	1	NC	1
155		2	max	0	9	0	2	.003	4	5.855e-6	9	NC	1	NC	1
156			min	0	2	-.001	3	0	9	-5.119e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	9	.002	2	.006	4	5.409e-6	9	NC	1	NC	1
158			min	0	2	-.003	3	0	9	-5.028e-4	4	NC	1	NC	1
159		4	max	0	9	.003	2	.008	4	4.963e-6	9	NC	1	NC	1
160			min	0	2	-.004	3	0	9	-4.938e-4	4	NC	1	NC	1
161		5	max	0	9	.003	2	.011	4	4.517e-6	9	NC	1	NC	1
162			min	0	2	-.005	3	0	9	-4.847e-4	4	NC	1	NC	1
163		6	max	0	9	.004	2	.014	4	4.071e-6	9	NC	1	NC	1
164			min	0	2	-.006	3	0	9	-4.757e-4	4	NC	1	NC	1
165		7	max	0	9	.005	2	.017	4	1.984e-5	3	NC	1	NC	1
166			min	0	2	-.008	3	0	9	-4.666e-4	4	8774.248	2	NC	1
167		8	max	0	9	.006	2	.019	4	3.682e-5	3	NC	1	NC	1
168			min	0	2	-.009	3	0	9	-4.575e-4	4	7404.3	2	NC	1
169		9	max	0	9	.007	2	.022	4	5.381e-5	3	NC	3	NC	1
170			min	0	2	-.01	3	0	9	-4.485e-4	4	6343.694	2	NC	1
171		10	max	0	9	.008	2	.025	4	7.079e-5	3	NC	3	NC	1
172			min	-.001	2	-.011	3	0	9	-4.394e-4	4	5497.53	2	NC	1
173		11	max	0	9	.01	2	.027	4	8.777e-5	3	NC	3	NC	1
174			min	-.001	2	-.012	3	0	9	-4.303e-4	4	4808.388	2	NC	1
175		12	max	0	9	.011	2	.029	4	1.048e-4	3	NC	3	NC	1
176			min	-.001	2	-.013	3	0	9	-4.213e-4	4	4238.975	2	NC	1
177		13	max	0	9	.012	2	.032	4	1.217e-4	3	NC	3	NC	1
178			min	-.001	2	-.014	3	0	9	-4.122e-4	4	3763.608	2	NC	1
179		14	max	0	9	.014	2	.034	4	1.387e-4	3	NC	3	NC	1
180			min	-.001	2	-.014	3	0	9	-4.032e-4	4	3363.743	2	NC	1
181		15	max	0	9	.015	2	.036	4	1.557e-4	3	NC	3	NC	1
182			min	-.002	2	-.015	3	0	9	-3.941e-4	4	3025.48	2	NC	1
183		16	max	0	9	.017	2	.038	4	1.727e-4	3	NC	3	NC	1
184			min	-.002	2	-.016	3	0	9	-3.85e-4	4	2738.109	2	NC	1
185		17	max	0	9	.018	2	.04	4	1.897e-4	3	NC	3	NC	1
186			min	-.002	2	-.016	3	0	9	-3.76e-4	4	2493.206	2	NC	1
187		18	max	0	9	.02	2	.041	4	2.066e-4	3	NC	3	NC	1
188			min	-.002	2	-.017	3	0	9	-3.669e-4	4	2284.056	2	NC	1
189		19	max	.001	9	.022	2	.043	4	2.236e-4	3	NC	3	NC	1
190			min	-.002	2	-.018	3	0	9	-3.578e-4	4	2105.255	2	NC	1
191	M8	1	max	.005	2	.018	2	0	9	1.574e-3	4	NC	1	NC	1
192			min	-.002	3	-.013	3	-.045	4	-1.771e-4	3	NC	1	427.785	4
193		2	max	.005	2	.017	2	0	9	1.574e-3	4	NC	1	NC	1
194			min	-.002	3	-.012	3	-.041	4	-1.771e-4	3	NC	1	466.279	4
195		3	max	.004	2	.016	2	0	9	1.574e-3	4	NC	1	NC	1
196			min	-.002	3	-.012	3	-.038	4	-1.771e-4	3	NC	1	512.087	4
197		4	max	.004	2	.015	2	0	9	1.574e-3	4	NC	1	NC	1
198			min	-.002	3	-.011	3	-.034	4	-1.771e-4	3	NC	1	567.133	4
199		5	max	.004	2	.014	2	0	9	1.574e-3	4	NC	1	NC	1
200			min	-.001	3	-.01	3	-.03	4	-1.771e-4	3	NC	1	634.042	4
201		6	max	.004	2	.013	2	0	9	1.574e-3	4	NC	1	NC	1
202			min	-.001	3	-.01	3	-.027	4	-1.771e-4	3	NC	1	716.462	4
203		7	max	.003	2	.012	2	0	9	1.574e-3	4	NC	1	NC	1
204			min	-.001	3	-.009	3	-.024	4	-1.771e-4	3	NC	1	819.588	4
205		8	max	.003	2	.011	2	0	9	1.574e-3	4	NC	1	NC	1
206			min	-.001	3	-.008	3	-.02	4	-1.771e-4	3	NC	1	951.026	4
207		9	max	.003	2	.01	2	0	9	1.574e-3	4	NC	1	NC	1
208			min	-.001	3	-.007	3	-.017	4	-1.771e-4	3	NC	1	1122.281	4
209		10	max	.002	2	.009	2	0	9	1.574e-3	4	NC	1	NC	1
210			min	0	3	-.007	3	-.014	4	-1.771e-4	3	NC	1	1351.435	4
211		11	max	.002	2	.008	2	0	9	1.574e-3	4	NC	1	NC	1
212			min	0	3	-.006	3	-.012	4	-1.771e-4	3	NC	1	1668.268	4
213		12	max	.002	2	.007	2	0	9	1.574e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	9	0	2	.004	4	5.47e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-4.608e-4	4	NC	1	NC	1
273		4	max	0	9	0	2	.006	4	3.796e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-4.956e-4	4	NC	1	NC	1
275		5	max	0	9	0	2	.009	4	2.121e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-5.305e-4	4	NC	1	NC	1
277		6	max	0	9	0	2	.011	4	4.465e-6	3	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-5.653e-4	4	NC	1	NC	1
279		7	max	0	9	0	2	.013	4	3.531e-6	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-6.002e-4	4	NC	1	NC	1
281		8	max	0	9	.001	2	.015	4	4.01e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-6.351e-4	4	NC	1	NC	1
283		9	max	0	9	.001	2	.018	4	4.49e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-6.699e-4	4	NC	1	NC	1
285		10	max	0	9	.002	2	.02	5	4.97e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-7.048e-4	4	NC	1	NC	1
287		11	max	0	9	.002	2	.022	5	5.449e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.002	3	-7.397e-4	4	NC	1	NC	1
289		12	max	0	9	.003	2	.024	5	5.929e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.002	3	-7.745e-4	4	NC	1	NC	1
291		13	max	0	9	.004	2	.026	5	6.408e-6	10	NC	1	NC	1
292			min	0	2	-.007	3	-.002	3	-8.094e-4	4	NC	1	NC	1
293		14	max	0	9	.004	2	.028	5	6.888e-6	10	NC	1	NC	1
294			min	0	2	-.007	3	-.002	3	-8.442e-4	4	NC	1	NC	1
295		15	max	0	9	.005	2	.03	5	7.367e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.002	3	-8.791e-4	4	8830.872	2	NC	1
297		16	max	0	9	.006	2	.032	5	7.847e-6	10	NC	1	NC	1
298			min	0	2	-.008	3	-.002	3	-9.14e-4	4	7525.807	2	NC	1
299		17	max	0	9	.007	2	.034	5	8.326e-6	10	NC	3	NC	1
300			min	0	2	-.008	3	-.002	3	-9.488e-4	4	6505.314	2	NC	1
301		18	max	0	9	.008	2	.036	5	8.806e-6	10	NC	3	NC	1
302			min	0	2	-.008	3	-.002	3	-9.837e-4	4	5700.181	2	NC	1
303		19	max	0	9	.009	2	.038	5	9.285e-6	10	NC	3	NC	1
304			min	0	2	-.008	3	-.002	3	-1.019e-3	4	5060.484	2	NC	1
305	M12	1	max	.002	1	.007	2	.001	1	2.131e-3	4	NC	1	NC	1
306			min	0	3	-.006	3	-.042	5	-9.052e-6	10	NC	1	463.472	5
307		2	max	.002	1	.007	2	.001	1	2.131e-3	4	NC	1	NC	1
308			min	0	3	-.005	3	-.038	5	-9.052e-6	10	NC	1	505.165	5
309		3	max	.001	1	.006	2	0	1	2.131e-3	4	NC	1	NC	1
310			min	0	3	-.005	3	-.035	5	-9.052e-6	10	NC	1	554.777	5
311		4	max	.001	1	.006	2	0	1	2.131e-3	4	NC	1	NC	1
312			min	0	3	-.005	3	-.031	5	-9.052e-6	10	NC	1	614.393	5
313		5	max	.001	1	.006	2	0	1	2.131e-3	4	NC	1	NC	1
314			min	0	3	-.004	3	-.028	5	-9.052e-6	10	NC	1	686.854	5
315		6	max	.001	1	.005	2	0	1	2.131e-3	4	NC	1	NC	1
316			min	0	3	-.004	3	-.025	5	-9.052e-6	10	NC	1	776.111	5
317		7	max	.001	1	.005	2	0	1	2.131e-3	4	NC	1	NC	1
318			min	0	3	-.004	3	-.022	5	-9.052e-6	10	NC	1	887.789	5
319		8	max	0	1	.004	2	0	1	2.131e-3	4	NC	1	NC	1
320			min	0	3	-.003	3	-.019	5	-9.052e-6	10	NC	1	1030.123	5
321		9	max	0	1	.004	2	0	1	2.131e-3	4	NC	1	NC	1
322			min	0	3	-.003	3	-.016	5	-9.052e-6	10	NC	1	1215.569	5
323		10	max	0	1	.004	2	0	1	2.131e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.013	5	-9.052e-6	10	NC	1	1463.708	5
325		11	max	0	1	.003	2	0	1	2.131e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.011	5	-9.052e-6	10	NC	1	1806.781	5
327		12	max	0	1	.003	2	0	1	2.131e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.008	5	-9.052e-6	10	NC	1	2301.025	5
329		13	max	0	1	.002	2	0	1	2.131e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.006	5	-9.052e-6	10	NC	1	3051.917	5
331		14	max	0	1	.002	2	0	1	2.131e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.005	5	-9.052e-6	10	NC	1	4277.005	5
333		15	max	0	1	.002	2	0	1	2.131e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.003	5	-9.052e-6	10	NC	1	6487.334	5
335		16	max	0	1	.001	2	0	1	2.131e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.002	5	-9.052e-6	10	NC	1	NC	1
337		17	max	0	1	0	2	0	1	2.131e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-9.052e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.131e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-9.052e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.131e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-9.052e-6	10	NC	1	NC	1
343	M1	1	max	.005	3	.024	3	.004	5	3.224e-3	1	NC	1	NC	1
344			min	-.006	2	-.02	2	0	9	-4.348e-3	3	NC	1	NC	1
345		2	max	.005	3	.013	3	.006	5	1.571e-3	1	NC	4	NC	1
346			min	-.006	2	-.01	2	0	9	-2.115e-3	3	4362.51	3	NC	1
347		3	max	.005	3	.002	3	.008	5	1.5e-4	5	NC	4	NC	1
348			min	-.006	2	-.002	1	-.001	1	-5.123e-5	9	2272.905	3	NC	1
349		4	max	.005	3	.007	2	.01	5	1.423e-4	5	NC	4	NC	1
350			min	-.006	2	-.006	3	-.001	1	-4.11e-5	9	1627.581	3	8536.401	5
351		5	max	.005	3	.014	2	.012	5	1.346e-4	5	NC	4	NC	1
352			min	-.006	2	-.013	3	-.002	1	-3.098e-5	9	1320.965	3	6017.059	5
353		6	max	.005	3	.02	2	.015	5	1.269e-4	5	NC	4	NC	1
354			min	-.006	2	-.019	3	-.001	1	-2.085e-5	9	1131.06	2	4568.275	5
355		7	max	.005	3	.024	2	.017	5	1.192e-4	5	NC	5	NC	1
356			min	-.006	2	-.023	3	-.001	1	-1.072e-5	9	1011.137	2	3642.856	5
357		8	max	.005	3	.027	2	.02	5	1.124e-4	4	NC	5	NC	1
358			min	-.006	2	-.026	3	0	9	-1.026e-6	10	936.518	2	3009.439	5
359		9	max	.005	3	.03	2	.023	5	1.079e-4	4	NC	5	NC	1
360			min	-.006	2	-.027	3	0	9	-1.953e-6	10	893.051	2	2552.495	4
361		10	max	.005	3	.03	2	.026	4	1.034e-4	4	NC	5	NC	1
362			min	-.006	2	-.027	3	0	9	-2.88e-6	10	874.033	2	2201.731	4
363		11	max	.005	3	.03	2	.028	4	9.884e-5	4	NC	5	NC	1
364			min	-.006	2	-.026	3	0	10	-3.807e-6	10	877.124	2	1935.421	4
365		12	max	.005	3	.028	2	.031	4	9.432e-5	4	NC	5	NC	1
366			min	-.006	2	-.024	3	0	10	-4.733e-6	10	903.46	2	1728.793	4
367		13	max	.005	3	.025	2	.034	4	8.98e-5	4	NC	5	NC	1
368			min	-.006	2	-.021	3	0	10	-5.66e-6	10	958.213	2	1565.806	4
369		14	max	.005	3	.02	2	.037	4	9.204e-5	1	NC	4	NC	1
370			min	-.006	2	-.017	3	0	10	-6.587e-6	10	1053.16	2	1435.692	4
371		15	max	.005	3	.014	2	.039	4	1.049e-4	1	NC	4	NC	1
372			min	-.006	2	-.011	3	0	10	-7.514e-6	10	1213.943	2	1331.014	4
373		16	max	.005	3	.006	2	.042	4	2.147e-4	4	NC	4	NC	1
374			min	-.006	2	-.005	3	0	10	-8.219e-6	10	1502.581	2	1246.509	4
375		17	max	.005	3	.002	3	.044	4	3.644e-3	4	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	-3.661e-6	10	2111.71	2	1178.476	4
377		18	max	.005	3	.01	3	.046	4	2.168e-3	2	NC	4	NC	1
378			min	-.006	2	-.014	2	0	10	-1.139e-3	3	4065.519	2	1123.963	4
379		19	max	.005	3	.018	3	.048	4	4.368e-3	2	NC	1	NC	1
380			min	-.006	2	-.026	2	0	9	-2.341e-3	3	NC	1	1082.148	4
381	M5	1	max	.012	3	.06	3	.004	5	1.001e-5	4	NC	1	NC	1
382			min	-.016	2	-.051	2	0	9	3.993e-8	9	NC	1	NC	1
383		2	max	.012	3	.033	3	.006	5	7.823e-5	3	NC	4	NC	1
384			min	-.016	2	-.027	2	0	9	-6.79e-6	9	1776.389	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.012	3	.007	3	.008	5	1.508e-4	3	NC	5	NC	1
386		min	-.016	2	-.005	1	0	9	-1.347e-5	9	920.278	3	NC	1
387	4	max	.012	3	.016	2	.01	5	1.479e-4	3	NC	5	NC	1
388		min	-.016	2	-.014	3	0	9	-1.245e-5	9	659.692	3	NC	1
389	5	max	.012	3	.033	2	.012	5	1.45e-4	3	NC	5	NC	1
390		min	-.016	2	-.031	3	0	9	-1.142e-5	9	536.365	3	NC	1
391	6	max	.012	3	.047	2	.015	5	1.426e-4	5	NC	5	NC	1
392		min	-.016	2	-.045	3	0	9	-1.039e-5	9	460.311	2	NC	1
393	7	max	.012	3	.058	2	.018	4	1.469e-4	5	NC	5	NC	1
394		min	-.016	2	-.055	3	0	9	-9.362e-6	9	410.873	2	NC	1
395	8	max	.012	3	.067	2	.021	4	1.513e-4	5	NC	5	NC	1
396		min	-.016	2	-.061	3	0	9	-8.334e-6	9	380.017	2	NC	1
397	9	max	.012	3	.072	2	.024	4	1.556e-4	5	NC	5	NC	1
398		min	-.016	2	-.065	3	0	9	-7.306e-6	9	361.912	2	NC	1
399	10	max	.012	3	.074	2	.027	4	1.599e-4	5	NC	5	NC	1
400		min	-.016	2	-.065	3	0	9	-6.278e-6	9	353.786	2	NC	1
401	11	max	.012	3	.073	2	.03	4	1.646e-4	4	NC	5	NC	1
402		min	-.016	2	-.063	3	0	9	-5.25e-6	9	354.656	2	NC	1
403	12	max	.012	3	.069	2	.033	4	1.694e-4	4	NC	5	NC	1
404		min	-.016	2	-.058	3	0	9	-4.222e-6	9	364.954	2	NC	1
405	13	max	.012	3	.061	2	.036	4	1.743e-4	4	NC	5	NC	1
406		min	-.016	2	-.05	3	0	9	-3.194e-6	9	386.752	2	NC	1
407	14	max	.012	3	.049	2	.039	4	1.791e-4	4	NC	5	NC	1
408		min	-.016	2	-.04	3	0	9	-2.166e-6	9	424.794	2	NC	1
409	15	max	.012	3	.034	2	.041	4	1.839e-4	4	NC	5	NC	1
410		min	-.016	2	-.027	3	0	9	-1.138e-6	9	489.443	2	NC	1
411	16	max	.012	3	.014	2	.043	4	3.243e-4	4	NC	5	NC	1
412		min	-.016	2	-.012	3	0	9	-6.998e-7	9	605.83	2	NC	1
413	17	max	.012	3	.005	3	.045	4	3.689e-3	4	NC	5	NC	1
414		min	-.016	2	-.009	2	0	9	-1.43e-5	9	852.461	2	NC	1
415	18	max	.012	3	.024	3	.047	4	1.895e-3	4	NC	4	NC	1
416		min	-.016	2	-.036	2	0	9	-7.378e-6	9	1652.249	2	NC	1
417	19	max	.012	3	.044	3	.048	4	4.452e-6	5	NC	1	NC	1
418		min	-.016	2	-.065	2	0	9	-5.541e-7	3	NC	1	NC	1
419	M9	1	max	.005	.023	3	.004	5	4.356e-3	3	NC	1	NC	1
420		min	-.006	2	-.02	2	0	9	-3.223e-3	1	NC	1	NC	1
421	2	max	.005	3	.012	3	.003	4	2.176e-3	3	NC	4	NC	1
422		min	-.006	2	-.011	2	0	10	-1.571e-3	1	4364.535	3	NC	1
423	3	max	.005	3	.002	3	.003	4	5.014e-5	1	NC	4	NC	1
424		min	-.006	2	-.002	1	0	3	-2.646e-5	5	2274.026	3	NC	1
425	4	max	.005	3	.007	2	.004	4	3.723e-5	1	NC	4	NC	1
426		min	-.006	2	-.006	3	-.001	3	-3.997e-5	5	1628.403	3	NC	1
427	5	max	.005	3	.014	2	.005	4	2.431e-5	1	NC	4	NC	1
428		min	-.006	2	-.013	3	-.002	3	-5.348e-5	5	1321.619	3	NC	1
429	6	max	.005	3	.019	2	.007	4	1.14e-5	1	NC	5	NC	1
430		min	-.006	2	-.019	3	-.003	3	-6.853e-5	4	1131.228	2	NC	1
431	7	max	.005	3	.024	2	.009	4	2.467e-7	10	NC	5	NC	1
432		min	-.006	2	-.023	3	-.003	3	-8.455e-5	4	1011.295	2	8875.408	4
433	8	max	.005	3	.027	2	.012	4	1.165e-6	10	NC	5	NC	1
434		min	-.006	2	-.026	3	-.003	3	-1.006e-4	4	936.67	2	6004.603	4
435	9	max	.005	3	.03	2	.014	4	2.083e-6	10	NC	5	NC	1
436		min	-.006	2	-.027	3	-.004	3	-1.166e-4	4	893.202	2	4400.804	4
437	10	max	.005	3	.03	2	.017	4	3.001e-6	10	NC	5	NC	1
438		min	-.006	2	-.027	3	-.004	3	-1.326e-4	4	874.186	2	3407.617	4
439	11	max	.005	3	.03	2	.021	5	3.92e-6	10	NC	5	NC	1
440		min	-.006	2	-.026	3	-.003	3	-1.486e-4	4	877.283	2	2747.587	4
441	12	max	.005	3	.028	2	.024	5	4.838e-6	10	NC	5	NC	1







Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

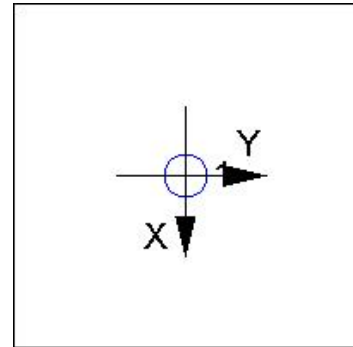
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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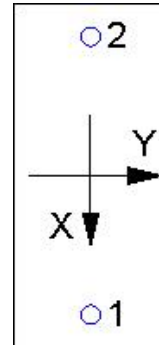
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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