

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	140 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 30.77 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

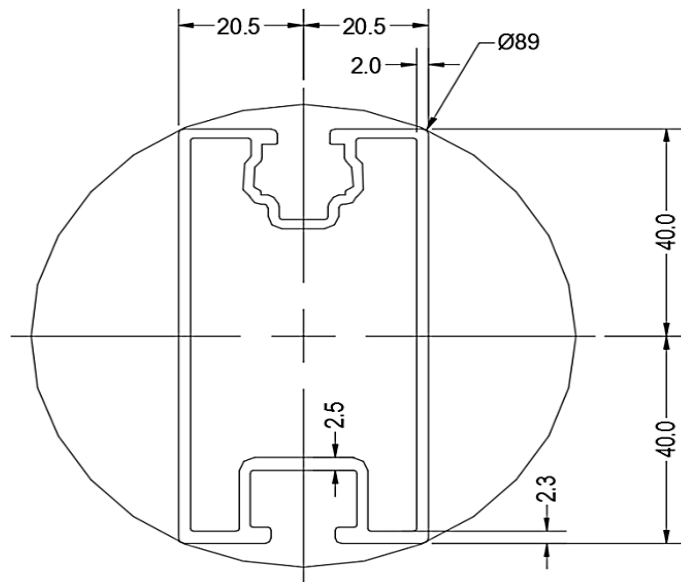
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

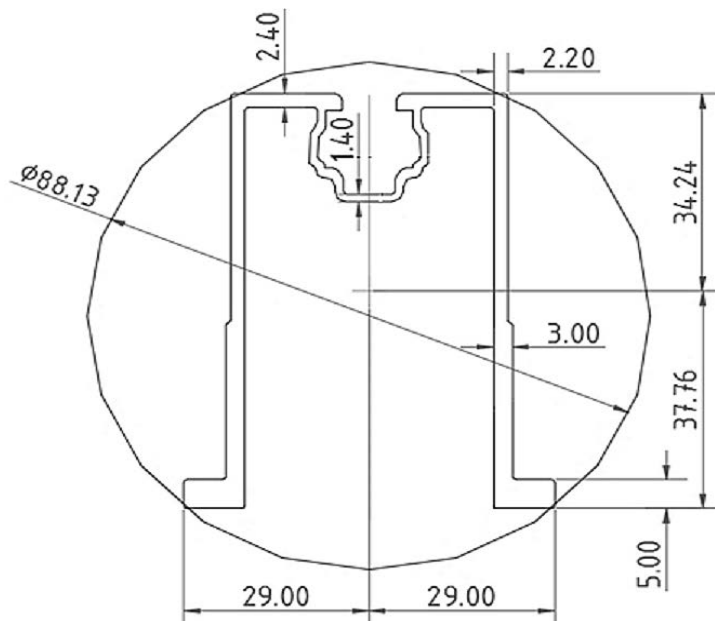
Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	60 in
$\Phi F_{ty}$ STRONG-AXIS =	29.75 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	0.487 k-ft
$M_z$ =	0.088 k-ft
$M_{y \text{ allowable}}$ =	1.848 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>37%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

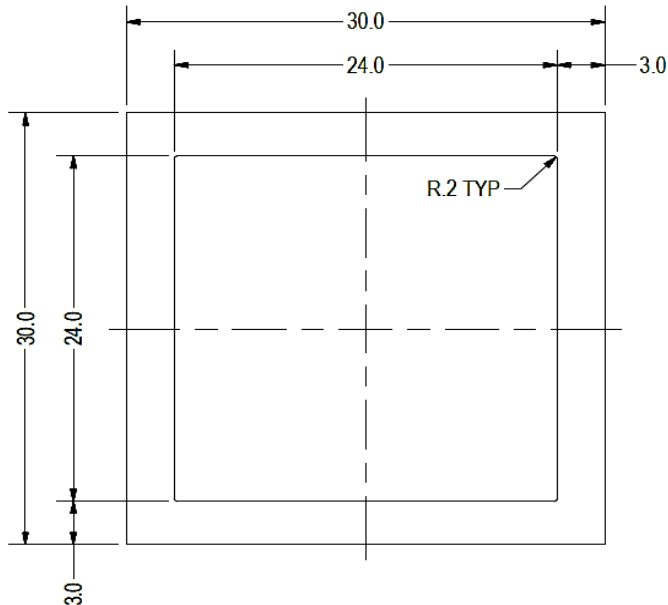
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.76 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.479 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.264 k
$M_{y \text{ allowable}}$ =	1.460 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>35%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.032 k-ft
$P_n$ =	0.191 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>9%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

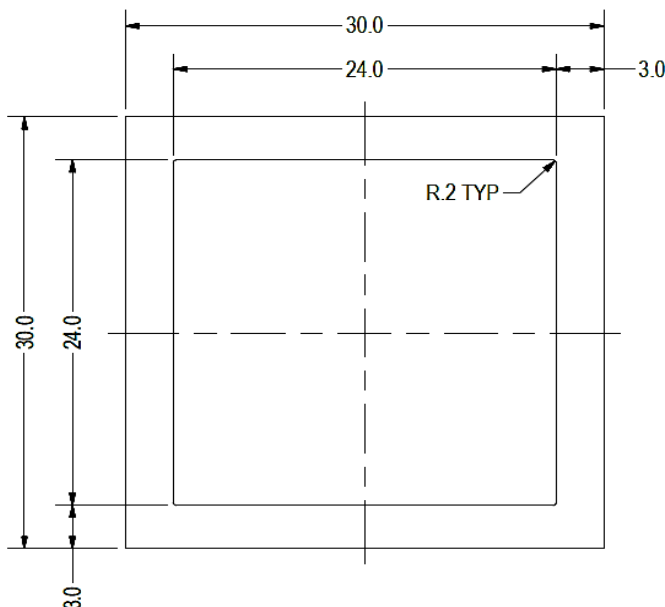
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.325 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>9%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.859 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	<b>13%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.189 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

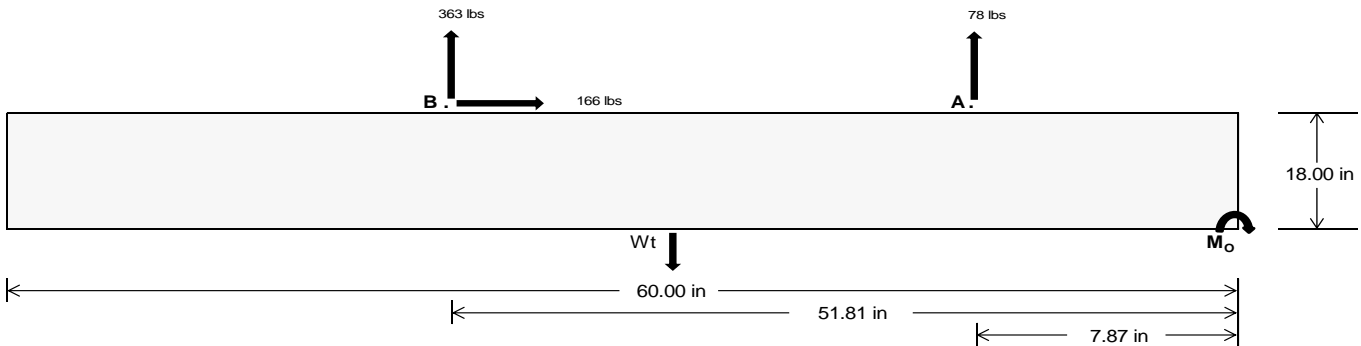
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>345.76</b>	<b>1577.06</b>	k
Compressive Load =	<b>1419.44</b>	<b>1102.15</b>	k
Lateral Load =	<b>26.19</b>	<b>719.40</b>	k
Moment (Weak Axis) =	<b>0.04</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 22410.1$  in-lbs  
Resisting Force Required = 747.00 lbs  
S.F. = 1.67  
Weight Required = 1245.01 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 165.96 lbs  
Friction = 0.4  
Weight Required = 414.90 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 165.96 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	481 lbs	481 lbs	481 lbs	481 lbs	496 lbs	496 lbs	496 lbs	496 lbs	696 lbs	696 lbs	696 lbs	696 lbs	-157 lbs	-157 lbs	-157 lbs	-157 lbs
$F_B$	344 lbs	344 lbs	344 lbs	344 lbs	437 lbs	437 lbs	437 lbs	437 lbs	558 lbs	558 lbs	558 lbs	558 lbs	-726 lbs	-726 lbs	-726 lbs	-726 lbs
$F_V$	34 lbs	34 lbs	34 lbs	34 lbs	294 lbs	294 lbs	294 lbs	294 lbs	243 lbs	243 lbs	243 lbs	243 lbs	-332 lbs	-332 lbs	-332 lbs	-332 lbs
$P_{total}$	2728 lbs	2818 lbs	2909 lbs	3000 lbs	2836 lbs	2926 lbs	3017 lbs	3108 lbs	3157 lbs	3248 lbs	3339 lbs	3429 lbs	259 lbs	314 lbs	368 lbs	422 lbs
$M$	312 lbs-ft	312 lbs-ft	312 lbs-ft	312 lbs-ft	561 lbs-ft	561 lbs-ft	561 lbs-ft	561 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	533 lbs-ft	533 lbs-ft	533 lbs-ft	533 lbs-ft
$e$	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.05 ft	1.70 ft	1.45 ft	1.26 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	269.0 psf	266.6 psf	264.5 psf	262.5 psf	247.1 psf	245.8 psf	244.6 psf	243.4 psf	274.0 psf	271.5 psf	269.1 psf	267.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	354.5 psf	348.3 psf	342.6 psf	337.4 psf	401.0 psf	392.7 psf	385.1 psf	378.1 psf	447.7 psf	437.2 psf	427.7 psf	418.9 psf	221.7 psf	142.2 psf	121.6 psf	113.6 psf

Maximum Bearing Pressure = 448 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

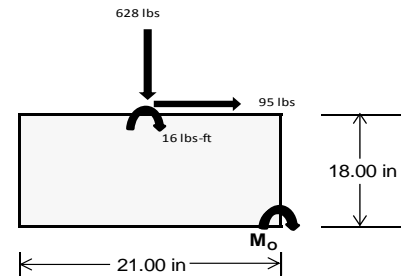
### Overturning Check

$M_o = 390.4 \text{ ft-lbs}$   
 Resisting Force Required = 446.18 lbs  
 S.F. = 1.67  
 Weight Required = 743.63 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	116 lbs	98 lbs	62 lbs	270 lbs	628 lbs	228 lbs	73 lbs	-13 lbs	21 lbs
$F_v$	15 lbs	127 lbs	15 lbs	10 lbs	95 lbs	11 lbs	15 lbs	127 lbs	15 lbs
$P_{total}$	2472 lbs	2454 lbs	2418 lbs	2512 lbs	2871 lbs	2470 lbs	762 lbs	676 lbs	710 lbs
$M$	42 lbs-ft	211 lbs-ft	43 lbs-ft	29 lbs-ft	159 lbs-ft	33 lbs-ft	42 lbs-ft	211 lbs-ft	43 lbs-ft
$e$	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.01 ft	0.06 ft	0.31 ft	0.06 ft
$L/6$	0.29 ft	1.58 ft	1.71 ft	1.73 ft	1.64 ft	1.72 ft	1.64 ft	1.13 ft	1.63 ft
$f_{min}$	266.1 sqft	197.7 sqft	259.4 sqft	275.6 sqft	265.7 sqft	269.5 sqft	70.6 sqft	-5.5 sqft	64.3 sqft
$f_{max}$	299.0 psf	363.2 psf	293.2 psf	298.6 psf	390.5 psf	295.2 psf	103.6 psf	159.9 psf	98.0 psf



Maximum Bearing Pressure = 391 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

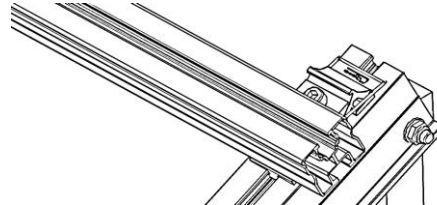
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.612 k
Allowable Uplift =	1.214 k
Utilization =	<u>50%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.086 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.092 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Diagonal Strut

Maximum Axial Load =	0.325 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



#### Rear Strut

Maximum Axial Load =	1.130 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.189 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	29.57 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.591 in
Max Drift, $\Delta_{MAX}$ =	0.066 in
	<u>0.066 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 60.00 \text{ in}$$

$$J = 0.427$$

$$125.139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 60.00 \text{ in}$$

$$J = 0.427$$

$$135.981$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.848 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &21.4323 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

### 3.4.15

N/A for Strong Direction

### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

### 3.4.16

N/A for Strong Direction

Weak Axis:

### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

### 3.4.16

N/A for Weak Direction

### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	271.982	1	.079	2	.261	1	0	10	0	4	0	15
30			min	-359.61	3	.014	15	-.442	5	0	4	0	3	0	6
31		16	max	272.089	1	.047	2	.261	1	0	10	0	1	0	15
32			min	-359.53	3	-.005	3	-.538	5	0	4	0	3	0	6
33		17	max	272.195	1	.014	2	.261	1	0	10	0	1	0	15
34			min	-359.451	3	-.029	3	-.635	5	0	4	0	3	0	6
35		18	max	272.302	1	-.015	15	.261	1	0	10	0	1	0	15
36			min	-359.371	3	-.059	4	-.731	5	0	4	0	3	0	6
37		19	max	272.408	1	-.025	15	.261	1	0	10	0	1	0	15
38			min	-359.291	3	-.1	4	-.828	5	0	4	0	3	0	6
39	M3	1	max	88.913	2	1.795	6	-.012	10	0	5	0	1	0	6
40			min	-85.209	3	.421	15	-1.382	4	0	1	0	10	0	15
41		2	max	88.845	2	1.617	6	-.012	10	0	5	0	1	0	6
42			min	-85.259	3	.379	15	-1.248	4	0	1	0	10	0	15
43		3	max	88.777	2	1.439	6	-.012	10	0	5	0	1	0	2
44			min	-85.31	3	.337	15	-1.114	4	0	1	0	10	0	3
45		4	max	88.709	2	1.262	6	-.012	10	0	5	0	1	0	15
46			min	-85.361	3	.295	15	-.981	4	0	1	0	5	0	4
47		5	max	88.641	2	1.084	6	-.012	10	0	5	0	1	0	15
48			min	-85.412	3	.254	15	-.847	4	0	1	0	5	0	4
49		6	max	88.573	2	.906	6	-.012	10	0	5	0	1	0	15
50			min	-85.463	3	.212	15	-.714	4	0	1	0	5	0	4
51		7	max	88.505	2	.729	6	-.012	10	0	5	0	1	0	15
52			min	-85.514	3	.17	15	-.58	4	0	1	0	5	0	4
53		8	max	88.438	2	.551	6	-.012	10	0	5	0	1	0	15
54			min	-85.565	3	.128	15	-.446	4	0	1	0	5	0	4
55		9	max	88.37	2	.373	6	-.012	10	0	5	0	1	0	15
56			min	-85.616	3	.087	15	-.313	4	0	1	0	5	-.001	4
57		10	max	88.302	2	.196	6	-.012	10	0	5	0	1	0	15
58			min	-85.667	3	.045	15	-.247	1	0	1	0	5	-.001	4
59		11	max	88.234	2	.034	2	.006	5	0	5	0	1	0	15
60			min	-85.718	3	-.003	3	-.247	1	0	1	0	5	-.001	4
61		12	max	88.166	2	-.039	15	.14	5	0	5	0	1	0	15
62			min	-85.768	3	-.16	4	-.247	1	0	1	0	5	-.001	4
63		13	max	88.098	2	-.08	15	.274	5	0	5	0	1	0	15
64			min	-85.819	3	-.337	4	-.247	1	0	1	0	5	-.001	4
65		14	max	88.03	2	-.122	15	.407	5	0	5	0	1	0	15
66			min	-85.87	3	-.515	4	-.247	1	0	1	0	5	-.001	4
67		15	max	87.962	2	-.164	15	.541	5	0	5	0	9	0	15
68			min	-85.921	3	-.693	4	-.247	1	0	1	0	5	0	4
69		16	max	87.895	2	-.206	15	.674	5	0	5	0	10	0	15
70			min	-85.972	3	-.87	4	-.247	1	0	1	0	4	0	4
71		17	max	87.827	2	-.247	15	.808	5	0	5	0	10	0	15
72			min	-86.023	3	-1.048	4	-.247	1	0	1	0	4	0	4
73		18	max	87.759	2	-.289	15	.942	5	0	5	0	10	0	15
74			min	-86.074	3	-1.226	4	-.247	1	0	1	0	4	0	4
75		19	max	87.691	2	-.331	15	1.075	5	0	5	0	5	0	1
76			min	-86.125	3	-1.403	4	-.247	1	0	1	0	1	0	1
77	M4	1	max	376.228	1	0	1	-.038	10	0	1	0	5	0	1
78			min	-74.77	3	0	1	-18.975	4	0	1	0	2	0	1
79		2	max	376.293	1	0	1	-.038	10	0	1	0	12	0	1
80			min	-74.721	3	0	1	-19.032	4	0	1	-.002	4	0	1
81		3	max	376.357	1	0	1	-.038	10	0	1	0	10	0	1
82			min	-74.673	3	0	1	-19.088	4	0	1	-.003	4	0	1
83		4	max	376.422	1	0	1	-.038	10	0	1	0	10	0	1
84			min	-74.624	3	0	1	-19.144	4	0	1	-.005	4	0	1
85		5	max	376.487	1	0	1	-.038	10	0	1	0	10	0	1

***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-74.576	3	0	1	-19.2	4	0	1	-.007	4	0	1
87		6	max	376.551	1	0	1	-.038	10	0	1	0	10	0	1
88			min	-74.527	3	0	1	-19.256	4	0	1	-.009	4	0	1
89		7	max	376.616	1	0	1	-.038	10	0	1	0	10	0	1
90			min	-74.479	3	0	1	-19.312	4	0	1	-.01	4	0	1
91		8	max	376.681	1	0	1	-.038	10	0	1	0	10	0	1
92			min	-74.43	3	0	1	-19.368	4	0	1	-.012	4	0	1
93		9	max	376.746	1	0	1	-.038	10	0	1	0	10	0	1
94			min	-74.382	3	0	1	-19.424	4	0	1	-.014	4	0	1
95		10	max	376.81	1	0	1	-.038	10	0	1	0	10	0	1
96			min	-74.333	3	0	1	-19.48	4	0	1	-.015	4	0	1
97		11	max	376.875	1	0	1	-.038	10	0	1	0	10	0	1
98			min	-74.285	3	0	1	-19.536	4	0	1	-.017	4	0	1
99		12	max	376.94	1	0	1	-.038	10	0	1	0	10	0	1
100			min	-74.236	3	0	1	-19.592	4	0	1	-.019	4	0	1
101		13	max	377.004	1	0	1	-.038	10	0	1	0	10	0	1
102			min	-74.187	3	0	1	-19.648	4	0	1	-.021	4	0	1
103		14	max	377.069	1	0	1	-.038	10	0	1	0	10	0	1
104			min	-74.139	3	0	1	-19.705	4	0	1	-.022	4	0	1
105		15	max	377.134	1	0	1	-.038	10	0	1	0	10	0	1
106			min	-74.09	3	0	1	-19.761	4	0	1	-.024	4	0	1
107		16	max	377.198	1	0	1	-.038	10	0	1	0	10	0	1
108			min	-74.042	3	0	1	-19.817	4	0	1	-.026	4	0	1
109		17	max	377.263	1	0	1	-.038	10	0	1	0	10	0	1
110			min	-73.993	3	0	1	-19.873	4	0	1	-.028	4	0	1
111		18	max	377.328	1	0	1	-.038	10	0	1	0	10	0	1
112			min	-73.945	3	0	1	-19.929	4	0	1	-.03	4	0	1
113		19	max	377.393	1	0	1	-.038	10	0	1	0	10	0	1
114			min	-73.896	3	0	1	-19.985	4	0	1	-.031	4	0	1
115	M6	1	max	857.196	1	.631	6	.94	4	0	3	0	3	0	1
116			min	-1129.711	3	.144	15	-.208	3	0	5	0	2	0	1
117		2	max	857.303	1	.589	6	.843	4	0	3	0	4	0	15
118			min	-1129.631	3	.134	15	-.208	3	0	5	0	2	0	6
119		3	max	857.409	1	.548	6	.747	4	0	3	0	4	0	15
120			min	-1129.551	3	.125	15	-.208	3	0	5	0	2	0	6
121		4	max	857.516	1	.507	6	.65	4	0	3	0	4	0	15





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	858.688	1	.132	2	.08	1	0	3	0	4	0	15
144		min	-1128.592	3	-.02	3	-.436	5	0	5	0	3	0	2
145	16	max	858.794	1	.1	2	.08	1	0	3	0	4	0	15
146		min	-1128.512	3	-.044	3	-.533	5	0	5	0	3	0	2
147	17	max	858.901	1	.068	2	.08	1	0	3	0	4	0	15
148		min	-1128.432	3	-.068	3	-.629	5	0	5	0	3	0	2
149	18	max	859.007	1	.036	2	.08	1	0	3	0	4	0	15
150		min	-1128.352	3	-.092	3	-.725	5	0	5	0	3	0	2
151	19	max	859.114	1	.003	2	.08	1	0	3	0	14	0	15
152		min	-1128.273	3	-.117	3	-.822	5	0	5	0	3	0	2
153	M7	1	max	324.786	2	1.805	.008	3	0	1	0	4	0	2
154		min	-242.328	3	.428	15	-1.412	4	0	3	0	3	0	12
155	2	max	324.718	2	1.627	4	.008	3	0	1	0	4	0	2
156		min	-242.379	3	.387	15	-1.278	4	0	3	0	3	0	12
157	3	max	324.65	2	1.449	4	.008	3	0	1	0	4	0	2
158		min	-242.429	3	.345	15	-1.145	4	0	3	0	3	0	3
159	4	max	324.582	2	1.272	4	.008	3	0	1	0	1	0	2
160		min	-242.48	3	.303	15	-1.011	4	0	3	0	3	0	3
161	5	max	324.514	2	1.094	4	.008	3	0	1	0	1	0	15
162		min	-242.531	3	.261	15	-.877	4	0	3	0	5	0	6
163	6	max	324.446	2	.916	4	.008	3	0	1	0	1	0	15
164		min	-242.582	3	.22	15	-.744	4	0	3	0	5	0	6
165	7	max	324.379	2	.739	4	.008	3	0	1	0	1	0	15
166		min	-242.633	3	.178	15	-.61	4	0	3	0	5	0	6
167	8	max	324.311	2	.561	4	.008	3	0	1	0	1	0	15
168		min	-242.684	3	.136	15	-.476	4	0	3	0	5	0	6
169	9	max	324.243	2	.383	4	.008	3	0	1	0	1	0	15
170		min	-242.735	3	.094	15	-.343	4	0	3	0	5	-.001	6
171	10	max	324.175	2	.211	2	.008	3	0	1	0	1	0	15
172		min	-242.786	3	.046	12	-.209	4	0	3	0	5	-.001	6
173	11	max	324.107	2	.073	2	.008	3	0	1	0	1	0	15
174		min	-242.837	3	-.039	3	-.076	4	0	3	0	5	-.001	6
175	12	max	324.039	2	-.031	15	.059	5	0	1	0	1	0	15
176		min	-242.888	3	-.15	6	-.008	1	0	3	0	5	-.001	6
177	13	max	323.971	2	-.073	15	.193	5	0	1	0	1	0	15
178		min	-242.938	3	-.327	6	-.008	1	0	3	0	5	-.001	6
179	14	max	323.904	2	-.115	15	.326	5	0	1	0	1	0	15
180		min	-242.989	3	-.505	6	-.008	1	0	3	0	5	-.001	6
181	15	max	323.836	2	-.156	15	.46	5	0	1	0	1	0	15
182		min	-243.04	3	-.683	6	-.008	1	0	3	0	5	0	6
183	16	max	323.768	2	-.198	15	.594	5	0	1	0	1	0	15
184		min	-243.091	3	-.86	6	-.008	1	0	3	0	5	0	6
185	17	max	323.7	2	-.24	15	.727	5	0	1	0	1	0	15
186		min	-243.142	3	-1.038	6	-.008	1	0	3	0	5	0	6
187	18	max	323.632	2	-.282	15	.861	5	0	1	0	1	0	15
188		min	-243.193	3	-1.216	6	-.008	1	0	3	0	5	0	6
189	19	max	323.564	2	-.323	15	.995	5	0	1	0	1	0	1
190		min	-243.244	3	-1.393	6	-.008	1	0	3	0	3	0	1
191	M8	1	max	1090.709	1	0	.324	1	0	1	0	4	0	1
192		min	-266.846	3	0	1	-19.325	4	0	1	0	1	0	1
193	2	max	1090.774	1	0	1	.324	1	0	1	0	1	0	1
194		min	-266.797	3	0	1	-19.381	4	0	1	-.002	4	0	1
195	3	max	1090.839	1	0	1	.324	1	0	1	0	1	0	1
196		min	-266.749	3	0	1	-19.437	4	0	1	-.003	4	0	1
197	4	max	1090.903	1	0	1	.324	1	0	1	0	1	0	1
198		min	-266.7	3	0	1	-19.493	4	0	1	-.005	4	0	1
199	5	max	1090.968	1	0	1	.324	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-266.652	3	0	1	-19.549	4	0	1	-.007	4	0	1
201		6	max	1091.033	1	0	1	.324	1	0	1	0	1	0	1
202			min	-266.603	3	0	1	-19.605	4	0	1	-.009	4	0	1
203		7	max	1091.097	1	0	1	.324	1	0	1	0	1	0	1
204			min	-266.555	3	0	1	-19.661	4	0	1	-.01	4	0	1
205		8	max	1091.162	1	0	1	.324	1	0	1	0	1	0	1
206			min	-266.506	3	0	1	-19.717	4	0	1	-.012	4	0	1
207		9	max	1091.227	1	0	1	.324	1	0	1	0	1	0	1
208			min	-266.457	3	0	1	-19.773	4	0	1	-.014	4	0	1
209		10	max	1091.291	1	0	1	.324	1	0	1	0	1	0	1
210			min	-266.409	3	0	1	-19.83	4	0	1	-.016	4	0	1
211		11	max	1091.356	1	0	1	.324	1	0	1	0	1	0	1
212			min	-266.36	3	0	1	-19.886	4	0	1	-.018	4	0	1
213		12	max	1091.421	1	0	1	.324	1	0	1	0	1	0	1
214			min	-266.312	3	0	1	-19.942	4	0	1	-.019	4	0	1
215		13	max	1091.486	1	0	1	.324	1	0	1	0	1	0	1
216			min	-266.263	3	0	1	-19.998	4	0	1	-.021	4	0	1
217		14	max	1091.55	1	0	1	.324	1	0	1	0	1	0	1
218			min	-266.215	3	0	1	-20.054	4	0	1	-.023	4	0	1
219		15	max	1091.615	1	0	1	.324	1	0	1	0	1	0	1
220			min	-266.166	3	0	1	-20.11	4	0	1	-.025	4	0	1
221		16	max	1091.68	1	0	1	.324	1	0	1	0	1	0	1
222			min	-266.118	3	0	1	-20.166	4	0	1	-.026	4	0	1
223		17	max	1091.744	1	0	1	.324	1	0	1	0	1	0	1
224			min	-266.069	3	0	1	-20.222	4	0	1	-.028	4	0	1
225		18	max	1091.809	1	0	1	.324	1	0	1	0	1	0	1
226			min	-266.021	3	0	1	-20.278	4	0	1	-.03	4	0	1
227		19	max	1091.874	1	0	1	.324	1	0	1	0	1	0	1
228			min	-265.972	3	0	1	-20.334	4	0	1	-.032	4	0	1
229	M10	1	max	272.503	1	.671	4	1.08	5	0	1	0	1	0	1
230			min	-329.609	3	.169	15	-.103	1	-.001	5	0	3	0	1
231		2	max	272.61	1	.63	4	.983	5	0	1	0	4	0	15
232			min	-329.529	3	.159	15	-.103	1	-.001	5	0	3	0	4
233		3	max	272.716	1	.589	4	.887	5	0	1	0	4	0	15
234			min	-329.449	3	.149	15	-.103	1	-.001	5	0	3	0	4
235		4	max	272.823	1	.548	4	.791	5	0	1	0	4	0	15
236			min	-329.369	3	.14	15	-.103	1	-.001	5	0	3	0	4
237		5	max	272.929	1	.506	4	.694	5	0	1	0	4	0	15
238			min	-329.29	3	.13	15	-.103	1	-.001	5	0	3	0	4
239		6	max	273.036	1	.465	4	.598	5	0	1	0	4	0	15
240			min	-329.21	3	.12	15	-.103	1	-.001	5	0	3	0	4
241		7	max	273.142	1	.424	4	.501	5	0	1	0	4	0	15
242			min	-329.13	3	.111	15	-.103	1	-.001	5	0	3	0	4
243		8	max	273.249	1	.382	4	.405	5	0	1	0	4	0	15
244			min	-329.05	3	.101	15	-.103	1	-.001	5	0	3	0	4
245		9	max	273.355	1	.341	4	.308	5	0	1	0	5	0	15
246			min	-328.97	3	.091	15	-.103	1	-.001	5	0	3	0	4
247		10	max	273.462	1	.3	4	.212	5	0	1	0	5	0	15
248			min	-328.89	3	.082	15	-.103	1	-.001	5	0	3	0	4
249		11	max	273.568	1	.259	4	.115	5	0	1	0	5	0	15
250			min	-328.81	3	.072	15	-.103	1	-.001	5	0	3	0	4
251		12	max	273.675	1	.217	4	.019	5	0	1	0	5	0	15
252			min	-328.73	3	.062	15	-.103	1	-.001	5	0	3	0	4
253		13	max	273.782	1	.176	4	-.01	10	0	1	0	5	0	15
254			min	-328.65	3	.052	15	-.103	1	-.001	5	0	3	0	4
255		14	max	273.888	1	.135	4	-.01	10	0	1	0	5	0	15
256			min	-328.57	3	.043	15	-.185	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	273.995	1	.094	4	-.01	10	0	1	0	5	0	15
258		min	-328.49	3	.033	15	-.282	4	-.001	5	0	3	0	4
259	16	max	274.101	1	.052	4	-.01	10	0	1	0	5	0	15
260		min	-328.411	3	.007	9	-.378	4	-.001	5	0	3	0	4
261	17	max	274.208	1	.02	5	-.01	10	0	1	0	5	0	15
262		min	-328.331	3	-.02	9	-.475	4	-.001	5	0	3	0	4
263	18	max	274.314	1	.005	5	-.01	10	0	1	0	5	0	15
264		min	-328.251	3	-.047	1	-.571	4	-.001	5	0	3	0	4
265	19	max	274.421	1	-.006	15	-.01	10	0	1	0	5	0	15
266		min	-328.171	3	-.079	1	-.668	4	-.001	5	0	3	0	4
267	M11	1	max	88.438	2	1.792	.273	1	0	4	.001	5	0	6
268		min	-85.823	3	.419	15	-1.251	5	0	10	0	1	0	15
269	2	max	88.37	2	1.614	6	.273	1	0	4	0	5	0	6
270		min	-85.874	3	.377	15	-1.117	5	0	10	0	1	0	15
271	3	max	88.302	2	1.436	6	.273	1	0	4	0	5	0	2
272		min	-85.925	3	.335	15	-.983	5	0	10	0	1	0	3
273	4	max	88.234	2	1.259	6	.273	1	0	4	0	5	0	15
274		min	-85.976	3	.293	15	-.85	5	0	10	0	1	0	4
275	5	max	88.166	2	1.081	6	.273	1	0	4	0	3	0	15
276		min	-86.027	3	.252	15	-.716	5	0	10	0	1	0	4
277	6	max	88.098	2	.903	6	.273	1	0	4	0	3	0	15
278		min	-86.078	3	.21	15	-.583	5	0	10	0	1	0	4
279	7	max	88.03	2	.726	6	.273	1	0	4	0	3	0	15
280		min	-86.129	3	.168	15	-.449	5	0	10	0	1	0	4
281	8	max	87.963	2	.548	6	.273	1	0	4	0	3	0	15
282		min	-86.18	3	.126	15	-.315	5	0	10	0	1	-.001	4
283	9	max	87.895	2	.37	6	.273	1	0	4	0	3	0	15
284		min	-86.23	3	.084	15	-.182	5	0	10	0	1	-.001	4
285	10	max	87.827	2	.193	6	.273	1	0	4	0	3	0	15
286		min	-86.281	3	.043	15	-.048	5	0	10	0	4	-.001	4
287	11	max	87.759	2	.034	2	.273	1	0	4	0	3	0	15
288		min	-86.332	3	-.019	3	-.017	3	0	10	0	4	-.001	4
289	12	max	87.691	2	-.041	15	.276	4	0	4	0	3	0	15
290		min	-86.383	3	-.163	4	-.017	3	0	10	0	4	-.001	4
291	13	max	87.623	2	-.083	15	.41	4	0	4	0	3	0	15
292		min	-86.434	3	-.341	4	-.017	3	0	10	0	4	-.001	4
293	14	max	87.555	2	-.124	15	.543	4	0	4	0	3	0	15
294		min	-86.485	3	-.518	4	-.017	3	0	10	0	5	-.001	4
295	15	max	87.488	2	-.166	15	.677	4	0	4	0	3	0	15
296		min	-86.536	3	-.696	4	-.017	3	0	10	0	10	0	4
297	16	max	87.42	2	-.208	15	.811	4	0	4	0	3	0	15
298		min	-86.587	3	-.873	4	-.017	3	0	10	0	10	0	4
299	17	max	87.352	2	-.25	15	.944	4	0	4	0	4	0	15
300		min	-86.638	3	-1.051	4	-.017	3	0	10	0	10	0	4
301	18	max	87.284	2	-.291	15	1.078	4	0	4	0	4	0	15
302		min	-86.688	3	-1.229	4	-.017	3	0	10	0	10	0	4
303	19	max	87.216	2	-.333	15	1.212	4	0	4	0	4	0	1
304		min	-86.739	3	-1.406	4	-.017	3	0	10	0	10	0	1
305	M12	1	max	376.226	1	0	1.281	1	0	1	0	4	0	1
306		min	-74.357	3	0	1	-17.727	5	0	1	0	3	0	1
307	2	max	376.291	1	0	1	1.281	1	0	1	0	1	0	1
308		min	-74.308	3	0	1	-17.783	5	0	1	-.002	5	0	1
309	3	max	376.356	1	0	1	1.281	1	0	1	0	1	0	1
310		min	-74.26	3	0	1	-17.839	5	0	1	-.003	5	0	1
311	4	max	376.42	1	0	1	1.281	1	0	1	0	1	0	1
312		min	-74.211	3	0	1	-17.895	5	0	1	-.005	5	0	1
313	5	max	376.485	1	0	1	1.281	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-74.163	3	0	1	-17.951	5	0	1	-.006	5	0	1
315	6	max	376.55	1	0	1	1.281	1	0	1	0	1	0	1
316		min	-74.114	3	0	1	-18.008	5	0	1	-.008	5	0	1
317	7	max	376.615	1	0	1	1.281	1	0	1	0	1	0	1
318		min	-74.066	3	0	1	-18.064	5	0	1	-.01	5	0	1
319	8	max	376.679	1	0	1	1.281	1	0	1	0	1	0	1
320		min	-74.017	3	0	1	-18.12	5	0	1	-.011	5	0	1
321	9	max	376.744	1	0	1	1.281	1	0	1	0	1	0	1
322		min	-73.969	3	0	1	-18.176	5	0	1	-.013	5	0	1
323	10	max	376.809	1	0	1	1.281	1	0	1	.001	1	0	1
324		min	-73.92	3	0	1	-18.232	5	0	1	-.014	5	0	1
325	11	max	376.873	1	0	1	1.281	1	0	1	.001	1	0	1
326		min	-73.872	3	0	1	-18.288	5	0	1	-.016	5	0	1
327	12	max	376.938	1	0	1	1.281	1	0	1	.001	1	0	1
328		min	-73.823	3	0	1	-18.344	5	0	1	-.018	5	0	1
329	13	max	377.003	1	0	1	1.281	1	0	1	.001	1	0	1
330		min	-73.775	3	0	1	-18.4	5	0	1	-.019	5	0	1
331	14	max	377.068	1	0	1	1.281	1	0	1	.002	1	0	1
332		min	-73.726	3	0	1	-18.456	5	0	1	-.021	5	0	1
333	15	max	377.132	1	0	1	1.281	1	0	1	.002	1	0	1
334		min	-73.678	3	0	1	-18.512	5	0	1	-.023	5	0	1
335	16	max	377.197	1	0	1	1.281	1	0	1	.002	1	0	1
336		min	-73.629	3	0	1	-18.568	5	0	1	-.024	5	0	1
337	17	max	377.262	1	0	1	1.281	1	0	1	.002	1	0	1
338		min	-73.581	3	0	1	-18.624	5	0	1	-.026	5	0	1
339	18	max	377.326	1	0	1	1.281	1	0	1	.002	1	0	1
340		min	-73.532	3	0	1	-18.68	5	0	1	-.028	5	0	1
341	19	max	377.391	1	0	1	1.281	1	0	1	.002	1	0	1
342		min	-73.483	3	0	1	-18.737	5	0	1	-.029	5	0	1
343	M1	1	max	81.107	1	339.52	3	-1.165	10	0	.051	1	.014	1
344		min	4.449	12	-273.675	1	-25.893	1	0	3	.002	10	-.015	3
345	2	max	81.202	1	339.323	3	-1.165	10	0	1	.045	1	.074	1
346		min	4.497	12	-273.938	1	-25.893	1	0	3	.002	10	-.089	3
347	3	max	66.635	1	5.115	14	-1.157	10	0	5	.039	1	.132	1
348		min	.802	10	-21.044	3	-25.737	1	0	1	.002	10	-.161	3
349	4	max	66.731	1	4.857	14	-1.157	10	0	5	.034	1	.133	1
350		min	.882	10	-21.24	3	-25.737	1	0	1	.002	10	-.156	3
351	5	max	66.826	1	4.599	14	-1.157	10	0	5	.028	1	.134	1
352		min	.961	10	-21.437	3	-25.737	1	0	1	.001	10	-.151	3
353	6	max	66.922	1	4.341	14	-1.157	10	0	5	.022	1	.135	1
354		min	1.041	10	-21.634	3	-25.737	1	0	1	.001	10	-.147	3
355	7	max	67.017	1	4.083	14	-1.157	10	0	5	.017	1	.136	1
356		min	1.121	10	-21.831	3	-25.737	1	0	1	0	10	-.142	3
357	8	max	67.113	1	3.826	14	-1.157	10	0	5	.011	1	.14	2
358		min	1.2	10	-22.028	3	-25.737	1	0	1	0	10	-.137	3
359	9	max	67.208	1	3.601	9	-1.157	10	0	5	.006	1	.144	2
360		min	1.28	10	-22.224	3	-25.737	1	0	1	0	10	-.133	3
361	10	max	67.304	1	3.383	9	-1.157	10	0	5	.001	3	.148	2
362		min	1.359	10	-22.421	3	-25.737	1	0	1	0	10	-.128	3
363	11	max	67.399	1	3.164	9	-1.157	10	0	5	0	3	.153	2
364		min	1.439	10	-22.618	3	-25.737	1	0	1	-.005	1	-.123	3
365	12	max	67.495	1	2.945	9	-1.157	10	0	5	0	12	.157	2
366		min	1.519	10	-22.815	3	-25.737	1	0	1	-.011	1	-.118	3
367	13	max	67.59	1	2.727	9	-1.157	10	0	5	0	12	.161	2
368		min	1.598	10	-23.012	3	-25.737	1	0	1	-.017	1	-.113	3
369	14	max	67.686	1	2.508	9	-1.157	10	0	5	0	10	.166	2
370		min	1.678	10	-23.208	3	-25.737	1	0	1	-.022	1	-.108	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	67.781	1	2.289	9	-1.157	10	0	5	-.001	10	.17	2
372			min	1.757	10	-23.405	3	-25.737	1	0	1	-.028	1	-.103	3
373		16	max	80.6	2	41.555	2	-1.168	10	0	1	-.002	10	.174	2
374			min	-30.618	3	-86.167	3	-25.96	1	0	5	-.034	1	-.097	3
375		17	max	80.695	2	41.293	2	-1.168	10	0	1	-.002	10	.165	2
376			min	-30.546	3	-86.364	3	-25.96	1	0	5	-.039	1	-.078	3
377		18	max	-3.229	12	344.713	2	-1.206	10	0	5	-.002	10	.092	2
378			min	-81.175	1	-156.898	3	-32.791	4	0	2	-.045	1	-.045	3
379		19	max	-3.181	12	344.451	2	-1.206	10	0	5	-.002	10	.017	2
380			min	-81.08	1	-157.094	3	-32.549	4	0	2	-.051	1	-.011	3
381	M5	1	max	191.737	1	1093.062	3	0	2	0	1	.036	4	.03	3
382			min	1.973	15	-877.607	1	-49.238	3	0	5	0	10	-.029	1
383		2	max	191.833	1	1092.865	3	0	2	0	1	.031	4	.162	1
384			min	2.002	15	-877.869	1	-49.238	3	0	5	-.004	3	-.207	3
385		3	max	144.648	1	6.605	9	5.31	3	0	3	.026	4	.349	1
386			min	.872	10	-66.708	3	-19.859	4	0	4	-.014	3	-.439	3
387		4	max	144.744	1	6.387	9	5.31	3	0	3	.022	4	.354	1
388			min	.951	10	-66.905	3	-19.617	4	0	4	-.013	3	-.425	3
389		5	max	144.839	1	6.168	9	5.31	3	0	3	.017	4	.358	1
390			min	1.031	10	-67.102	3	-19.375	4	0	4	-.011	3	-.41	3
391		6	max	144.935	1	5.949	9	5.31	3	0	3	.013	4	.363	1
392			min	1.111	10	-67.299	3	-19.133	4	0	4	-.01	3	-.396	3
393		7	max	145.03	1	5.731	9	5.31	3	0	3	.009	4	.368	1
394			min	1.19	10	-67.496	3	-18.891	4	0	4	-.009	3	-.381	3
395		8	max	145.126	1	5.512	9	5.31	3	0	3	.005	4	.375	2
396			min	1.27	10	-67.692	3	-18.649	4	0	4	-.008	3	-.366	3
397		9	max	145.221	1	5.293	9	5.31	3	0	3	.001	4	.388	2
398			min	1.309	15	-67.889	3	-18.407	4	0	4	-.007	3	-.352	3
399		10	max	145.317	1	5.075	9	5.31	3	0	3	0	2	.401	2
400			min	1.337	15	-68.086	3	-18.165	4	0	4	-.006	3	-.337	3
401		11	max	145.412	1	4.856	9	5.31	3	0	3	0	2	.414	2
402			min	1.366	15	-68.283	3	-17.923	4	0	4	-.007	4	-.322	3
403		12	max	145.508	1	4.637	9	5.31	3	0	3	0	2	.427	2
404			min	1.395	15	-68.48	3	-17.681	4	0	4	-.011	4	-.307	3
405		13	max	145.604	1	4.419	9	5.31	3	0	3	0	2	.441	2
406			min	1.424	15	-68.676	3	-17.439	4	0	4	-.014	4	-.292	3
407		14	max	145.699	1	4.2	9	5.31	3	0	3	0	2	.454	2
408			min	1.453	15	-68.873	3	-17.197	4	0	4	-.018	4	-.277	3
409		15	max	145.795	1	3.981	9	5.31	3	0	3	0	3	.467	2
410			min	1.481	15	-69.07	3	-16.955	4	0	4	-.022	4	-.262	3
411		16	max	263.619	2	173.383	2	5.285	3	0	3	0	3	.479	2
412			min	-98.155	3	-244.471	3	-15.725	4	0	4	-.026	4	-.246	3
413		17	max	263.714	2	173.12	2	5.285	3	0	3	.002	3	.441	2
414			min	-98.084	3	-244.668	3	-15.483	4	0	4	-.029	4	-.193	3
415		18	max	-4.807	12	1103.684	2	4.884	3	0	4	.003	3	.205	2
416			min	-191.889	1	-496.333	3	-34.082	5	0	1	-.036	4	-.086	3
417		19	max	-4.759	12	1103.421	2	4.884	3	0	4	.004	3	.021	3
418			min	-191.794	1	-496.529	3	-33.84	5	0	1	-.044	4	-.034	2
419	M9	1	max	80.877	1	339.484	3	141.821	4	0	3	0	15	.014	1
420			min	.775	15	-273.674	1	1.166	10	0	1	-.05	1	-.015	3
421		2	max	80.973	1	339.287	3	142.063	4	0	3	.029	5	.074	1
422			min	.804	15	-273.937	1	1.166	10	0	1	-.045	1	-.089	3
423		3	max	66.82	1	4.892	9	25.167	1	0	1	.057	5	.132	1
424			min	.754	15	-20.975	3	-25.563	5	0	10	-.038	1	-.161	3
425		4	max	66.916	1	4.673	9	25.167	1	0	1	.051	5	.133	1
426			min	.783	15	-21.172	3	-25.321	5	0	10	-.033	1	-.156	3
427		5	max	67.011	1	4.455	9	25.167	1	0	1	.046	5	.134	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	42.189	4	42.702	1	10.329	4	.015	3	0	5	.317	3
486			min	1.166	10	-54.277	3	-.809	10	-.014	1	-.042	1	-.256	1
487		16	max	36.689	4	44.166	3	22.149	1	.015	3	.006	5	.32	3
488			min	1.166	10	-36.32	1	1	10	-.014	1	-.035	1	-.258	1
489		17	max	31.188	4	142.609	3	41.8	1	.015	3	.013	5	.268	3
490			min	1.166	10	-115.342	1	2.809	10	-.014	1	-.017	1	-.216	1
491		18	max	25.943	1	241.051	3	61.452	1	.015	3	.022	4	.161	3
492			min	1.166	10	-194.365	1	3.81	12	-.014	1	0	10	-.13	1
493		19	max	25.943	1	339.494	3	81.103	1	.015	3	.051	1	0	1
494			min	1.166	10	-273.387	1	4.449	12	-.014	1	.002	10	0	3
495	M16	1	max	38.235	5	344.578	2	7.339	5	.011	3	.05	1	0	2
496			min	-26.632	1	-157.108	3	-80.852	1	-.017	2	-.031	5	0	3
497		2	max	32.734	5	244.938	2	8.352	5	.011	3	.011	1	.075	3
498			min	-26.632	1	-112.063	3	-61.201	1	-.017	2	-.027	5	-.164	2
499		3	max	27.233	5	145.297	2	9.365	5	.011	3	0	12	.125	3
500			min	-26.632	1	-67.019	3	-41.55	1	-.017	2	-.025	4	-.272	2
501		4	max	21.732	5	45.657	2	10.378	5	.011	3	-.002	12	.149	3
502			min	-26.632	1	-21.974	3	-21.899	1	-.017	2	-.035	1	-.325	2
503		5	max	16.231	5	23.07	3	11.391	5	.011	3	-.002	12	.149	3
504			min	-26.632	1	-53.984	2	-2.247	1	-.017	2	-.042	1	-.323	2
505		6	max	10.73	5	68.115	3	17.404	1	.011	3	-.002	15	.124	3
506			min	-26.632	1	-153.624	2	-.699	3	-.017	2	-.038	1	-.265	2
507		7	max	5.229	5	113.159	3	37.055	1	.011	3	.004	5	.073	3
508			min	-26.632	1	-253.265	2	.259	3	-.017	2	-.023	1	-.152	2
509		8	max	1.415	3	158.204	3	56.706	1	.011	3	.012	4	.016	2
510			min	-26.632	1	-352.905	2	.912	12	-.017	2	-.004	3	-.002	3
511		9	max	1.415	3	203.248	3	76.357	1	.011	3	.04	1	.24	2
512			min	-26.632	1	-452.546	2	1.551	12	-.017	2	-.003	3	-.103	3
513		10	max	22.576	5	-9.958	15	96.009	1	.005	14	.088	1	.519	2
514			min	-26.632	1	-552.186	2	-3.771	3	-.017	2	.002	12	-.228	3
515		11	max	17.075	5	452.546	2	4.774	5	.017	2	.04	1	.24	2
516			min	-26.551	1	-203.248	3	-76.125	1	-.011	3	-.013	5	-.103	3
517		12	max	11.574	5	352.905	2	5.787	5	.017	2	.004	2	.016	2
518			min	-26.551	1	-158.204	3	-56.474	1	-.011	3	-.01	5	-.002	3
519		13	max	6.073	5	253.265	2	6.8	5	.017	2	0	12	.073	3
520			min	-26.551	1	-113.159	3	-36.823	1	-.011	3	-.023	1	-.152	2
521		14	max	.572	5	153.624	2	7.813	5	.017	2	-.001	12	.124	3
522			min	-26.551	1	-68.115	3	-17.172	1	-.011	3	-.038	1	-.265	2
523		15	max	-1.205	10	53.984	2	9.578	4	.017	2	.003	5	.149	3
524			min	-26.551	1	-23.07	3	-.811	10	-.011	3	-.042	1	-.323	2
525		16	max	-1.205	10	21.974	3	22.131	1	.017	2	.008	5	.149	3
526			min	-26.551	1	-45.657	2	.998	10	-.011	3	-.035	1	-.325	2
527		17	max	-1.205	10	67.019	3	41.782	1	.017	2	.014	5	.125	3
528			min	-26.551	1	-145.297	2	1.903	12	-.011	3	-.017	1	-.272	2
529		18	max	-1.205	10	112.063	3	61.433	1	.017	2	.023	4	.075	3
530			min	-27.075	4	-244.938	2	2.542	12	-.011	3	0	10	-.164	2
531		19	max	-1.205	10	157.108	3	81.085	1	.017	2	.051	1	0	2
532			min	-32.576	4	-344.578	2	3.181	12	-.011	3	.002	10	0	5
533	M15	1	max	0	1	1.025	3	.08	3	0	1	0	1	0	1
534			min	-62.194	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.911	3	.08	3	0	1	0	1	0	1
536			min	-62.254	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.798	3	.08	3	0	1	0	1	0	1
538			min	-62.314	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.684	3	.08	3	0	1	0	1	0	1
540			min	-62.373	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.57	3	.08	3	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-62.433	3	0	1	0	1	0	3	0	3	-.001	3
543		6	max	0	1	.456	3	.08	3	0	1	0	1	0	1
544			min	-62.493	3	0	1	0	1	0	3	0	3	-.001	3
545		7	max	0	1	.342	3	.08	3	0	1	0	3	0	1
546			min	-62.552	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.228	3	.08	3	0	1	0	3	0	1
548			min	-62.612	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.114	3	.08	3	0	1	0	3	0	1
550			min	-62.672	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.08	3	0	1	0	3	0	1
552			min	-62.731	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.08	3	0	1	0	3	0	1
554			min	-62.791	3	-.114	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.08	3	0	1	0	3	0	1
556			min	-62.851	3	-.228	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.08	3	0	1	0	3	0	1
558			min	-62.91	3	-.342	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.08	3	0	1	0	3	0	1
560			min	-62.97	3	-.456	3	0	1	0	3	0	1	-.001	3
561		15	max	0	1	0	1	.08	3	0	1	0	3	0	1
562			min	-63.03	3	-.57	3	0	1	0	3	0	1	-.001	3
563		16	max	0	1	0	1	.08	3	0	1	0	3	0	1
564			min	-63.089	3	-.684	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.08	3	0	1	0	3	0	1
566			min	-63.149	3	-.798	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.08	3	0	1	0	3	0	1
568			min	-63.209	3	-.911	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.08	3	0	1	0	3	0	1
570			min	-63.268	3	-1.025	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.367	4	.246	4	0	3	0	3	0	1
572			min	-188.543	4	0	2	-.036	3	0	1	0	4	0	1
573		2	max	0	2	2.104	4	.222	4	0	3	0	3	0	2
574			min	-188.571	4	0	2	-.036	3	0	1	0	4	0	4
575		3	max	0	2	1.841	4	.199	4	0	3	0	3	0	2
576			min	-188.599	4	0	2	-.036	3	0	1	0	4	-.001	4
577		4	max	0	2	1.578	4	.175	4	0	3	0	3	0	2
578			min	-188.626	4	0	2	-.036	3	0	1	0	1	-.002	4
579		5	max	0	2	1.315	4	.152	4	0	3	0	3	0	2
580			min	-188.654	4	0	2	-.036	3	0	1	0	1	-.002	4
581		6	max	0	2	1.052	4	.128	4	0	3	0	3	0	2
582			min	-188.682	4	0	2	-.036	3	0	1	0	1	-.003	4
583		7	max	0	2	.789	4	.105	4	0	3	0	3	0	2
584			min	-188.71	4	0	2	-.036	3	0	1	0	1	-.003	4
585		8	max	0	2	.526	4	.081	4	0	3	0	5	0	2
586			min	-188.738	4	0	2	-.036	3	0	1	0	1	-.003	4
587		9	max	0	2	.263	4	.058	4	0	3	0	5	0	2
588			min	-188.766	4	0	2	-.036	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.037	1	0	3	0	5	0	2
590			min	-188.794	4	0	1	-.036	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.037	1	0	3	0	5	0	2
592			min	-188.822	4	-.263	4	-.036	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.037	1	0	3	0	5	0	2
594			min	-188.85	4	-.526	4	-.036	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.037	1	0	3	0	5	0	2
596			min	-188.878	4	-.789	4	-.04	5	0	1	0	9	-.003	4
597		14	max	0	2	0	2	.037	1	0	3	0	5	0	2
598			min	-188.905	4	-1.052	4	-.064	5	0	1	0	3	-.003	4





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	2	0	2	.037	1	0	3	0	4	0	2
600		min	-188.933	4	-1.315	4	-.087	5	0	1	0	3	-.002	4
601	16	max	0	2	0	2	.037	1	0	3	0	4	0	2
602		min	-188.961	4	-1.578	4	-.111	5	0	1	0	3	-.002	4
603	17	max	0	2	0	2	.037	1	0	3	0	1	0	2
604		min	-188.989	4	-1.841	4	-.134	5	0	1	0	3	-.001	4
605	18	max	.02	11	0	2	.037	1	0	3	0	1	0	2
606		min	-189.017	4	-2.104	4	-.158	5	0	1	0	3	0	4
607	19	max	.086	11	0	2	.037	1	0	3	0	1	0	1
608		min	-189.045	4	-2.367	4	-.181	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.007	2	.005	1	1.065e-3	5	NC	3	NC	2	
2			min	-.003	3	-.007	3	-.011	5	-3.801e-4	1	4450.655	2	6816.36	1	
3		2	max	.002	1	.007	2	.005	1	1.086e-3	5	NC	3	NC	2	
4			min	-.003	3	-.006	3	-.01	5	-3.645e-4	1	4827.564	2	7379.16	1	
5		3	max	.002	1	.006	2	.004	1	1.108e-3	5	NC	3	NC	2	
6			min	-.003	3	-.006	3	-.01	5	-3.489e-4	1	5270.745	2	8042.172	1	
7		4	max	.002	1	.006	2	.004	1	1.13e-3	5	NC	3	NC	2	
8			min	-.002	3	-.006	3	-.01	5	-3.332e-4	1	5795.359	2	8829.793	1	
9		5	max	.002	1	.005	2	.003	1	1.152e-3	5	NC	1	NC	2	
10			min	-.002	3	-.006	3	-.009	5	-3.176e-4	1	6421.382	2	9774.604	1	
11		6	max	.002	1	.005	2	.003	1	1.174e-3	5	NC	1	NC	1	
12			min	-.002	3	-.005	3	-.009	5	-3.02e-4	1	7175.555	2	NC	1	
13		7	max	.001	1	.004	2	.003	1	1.196e-3	5	NC	1	NC	1	
14			min	-.002	3	-.005	3	-.008	5	-2.864e-4	1	8094.354	2	NC	1	
15		8	max	.001	1	.004	2	.002	1	1.217e-3	5	NC	1	NC	1	
16			min	-.002	3	-.005	3	-.008	5	-2.707e-4	1	9228.621	2	NC	1	
17		9	max	.001	1	.003	2	.002	1	1.239e-3	5	NC	1	NC	1	
18			min	-.002	3	-.004	3	-.007	5	-2.551e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.002	1	1.261e-3	5	NC	1	NC	1	
20			min	-.001	3	-.004	3	-.007	5	-2.395e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	.001	1	1.283e-3	5	NC	1	NC	1	
22			min	-.001	3	-.004	3	-.006	5	-2.239e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.001	1	1.305e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.005	5	-2.082e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	1.327e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.005	5	-1.926e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.349e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.004	5	-1.77e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.37e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.003	5	-1.614e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.392e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-1.458e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.414e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-1.301e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.436e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-1.145e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.458e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-9.888e-5	1	NC	1	NC	1	
39		M3	1	max	0	1	0	1	0	1	4.544e-5	1	NC	1	NC	1
40				min	0	1	0	1	0	1	-6.701e-4	5	NC	1	NC	1
41	2		max	0	3	0	2	.004	5	5.801e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-6.734e-4	5	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	7.058e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.767e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.011	5	8.315e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.8e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	9.572e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.833e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.018	4	1.083e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-6.866e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.021	4	1.209e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-6.899e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.025	4	1.334e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-6.932e-4	5	NC	1	NC	1
55		9	max	0	3	.002	2	.028	4	1.46e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	-6.965e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.031	4	1.586e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-6.998e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.035	4	1.711e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-7.03e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.038	4	1.837e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-7.063e-4	5	NC	1	NC	1
63		13	max	0	3	.004	2	.041	4	1.963e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-7.096e-4	5	NC	1	NC	1
65		14	max	0	3	.005	2	.044	4	2.088e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-7.129e-4	5	NC	1	NC	1
67		15	max	0	3	.005	2	.047	4	2.214e-4	1	NC	1	NC	1
68			min	0	2	-.008	3	0	10	-7.162e-4	5	8574.794	2	NC	1
69		16	max	0	3	.006	2	.05	4	2.34e-4	1	NC	1	NC	1
70			min	0	2	-.008	3	0	10	-7.195e-4	5	7317.086	2	NC	1
71		17	max	0	3	.007	2	.052	4	2.466e-4	1	NC	3	NC	1
72			min	0	2	-.008	3	0	10	-7.228e-4	5	6332.053	2	NC	1
73		18	max	0	3	.008	2	.055	4	2.591e-4	1	NC	3	NC	1
74			min	0	2	-.008	3	0	10	-7.261e-4	5	5553.706	2	NC	1
75		19	max	0	3	.009	2	.058	4	2.717e-4	1	NC	3	NC	1
76			min	-.001	2	-.008	3	0	10	-7.294e-4	5	4934.384	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	2.919e-3	5	NC	1	NC	2
78			min	0	3	-.007	3	-.061	4	-3.266e-4	1	NC	1	316.426	4
79		2	max	.002	1	.008	2	0	10	2.919e-3	5	NC	1	NC	1
80			min	0	3	-.006	3	-.056	4	-3.266e-4	1	NC	1	344.923	4
81		3	max	.002	1	.008	2	0	10	2.919e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.051	4	-3.266e-4	1	NC	1	378.838	4
83		4	max	.001	1	.007	2	0	10	2.919e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.046	4	-3.266e-4	1	NC	1	419.598	4
85		5	max	.001	1	.007	2	0	10	2.919e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.041	4	-3.266e-4	1	NC	1	469.146	4
87		6	max	.001	1	.006	2	0	10	2.919e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.036	4	-3.266e-4	1	NC	1	530.185	4
89		7	max	.001	1	.006	2	0	10	2.919e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.032	4	-3.266e-4	1	NC	1	606.565	4
91		8	max	.001	1	.005	2	0	10	2.919e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.027	4	-3.266e-4	1	NC	1	703.923	4
93		9	max	0	1	.005	2	0	10	2.919e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.023	4	-3.266e-4	1	NC	1	830.782	4
95		10	max	0	1	.004	2	0	10	2.919e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.019	4	-3.266e-4	1	NC	1	1000.545	4
97		11	max	0	1	.004	2	0	10	2.919e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.016	4	-3.266e-4	1	NC	1	1235.28	4
99		12	max	0	1	.003	2	0	10	2.919e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.003	3	-.012	4	-3.266e-4	1	NC	1	1573.483	4
101		max	0	1	.003	2	0	10	2.919e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.009	4	-3.266e-4	1	NC	1	2087.356	4
103		max	0	1	.002	2	0	10	2.919e-3	5	NC	1	NC	1
104		min	0	3	-.002	3	-.007	4	-3.266e-4	1	NC	1	2925.83	4
105		max	0	1	.002	2	0	10	2.919e-3	5	NC	1	NC	1
106		min	0	3	-.002	3	-.004	4	-3.266e-4	1	NC	1	4438.778	4
107		max	0	1	.001	2	0	10	2.919e-3	5	NC	1	NC	1
108		min	0	3	-.001	3	-.003	4	-3.266e-4	1	NC	1	7620.778	4
109		max	0	1	0	2	0	10	2.919e-3	5	NC	1	NC	1
110		min	0	3	0	3	-.001	4	-3.266e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.919e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-3.266e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.919e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-3.266e-4	1	NC	1	NC	1
115	M6	max	.007	1	.021	2	.002	1	1.145e-3	4	NC	3	NC	1
116		min	-.009	3	-.017	3	-.011	5	-8.042e-8	2	1584.24	2	8690.372	3
117		max	.007	1	.02	2	.002	1	1.166e-3	4	NC	3	NC	1
118		min	-.009	3	-.016	3	-.01	5	-7.602e-8	2	1693.802	2	9296.8	3
119		max	.006	1	.018	2	.002	1	1.187e-3	4	NC	3	NC	1
120		min	-.008	3	-.015	3	-.01	5	-7.162e-8	2	1819.188	2	NC	1
121		max	.006	1	.017	2	.001	1	1.208e-3	4	NC	3	NC	1
122		min	-.008	3	-.015	3	-.01	5	-3.104e-7	11	1963.585	2	NC	1
123		max	.005	1	.016	2	.001	1	1.229e-3	4	NC	3	NC	1
124		min	-.007	3	-.014	3	-.009	5	-2.267e-6	1	2131.101	2	NC	1
125		max	.005	1	.014	2	.001	1	1.25e-3	4	NC	3	NC	1
126		min	-.007	3	-.013	3	-.009	5	-5.594e-6	1	2327.117	2	NC	1
127		max	.005	1	.013	2	.001	1	1.271e-3	4	NC	3	NC	1
128		min	-.006	3	-.012	3	-.008	5	-8.921e-6	1	2558.82	2	NC	1
129		max	.004	1	.012	2	0	1	1.292e-3	4	NC	3	NC	1
130		min	-.006	3	-.011	3	-.008	5	-1.225e-5	1	2836.017	2	NC	1
131		max	.004	1	.01	2	0	1	1.313e-3	4	NC	3	NC	1
132		min	-.005	3	-.01	3	-.007	5	-1.557e-5	1	3172.451	2	NC	1
133		max	.004	1	.009	2	0	1	1.334e-3	4	NC	3	NC	1
134		min	-.005	3	-.009	3	-.007	5	-1.89e-5	1	3587.981	2	NC	1
135		max	.003	1	.008	2	0	1	1.355e-3	4	NC	3	NC	1
136		min	-.004	3	-.008	3	-.006	5	-2.223e-5	1	4112.411	2	NC	1
137		max	.003	1	.007	2	0	1	1.376e-3	4	NC	3	NC	1
138		min	-.004	3	-.007	3	-.005	5	-2.556e-5	1	4792.58	2	NC	1
139		max	.002	1	.006	2	0	1	1.397e-3	4	NC	3	NC	1
140		min	-.003	3	-.006	3	-.005	5	-2.888e-5	1	5706.568	2	NC	1
141		max	.002	1	.005	2	0	1	1.418e-3	4	NC	3	NC	1
142		min	-.003	3	-.005	3	-.004	5	-3.221e-5	1	6994.925	2	NC	1
143		max	.002	1	.004	2	0	1	1.439e-3	4	NC	1	NC	1
144		min	-.002	3	-.004	3	-.003	5	-3.554e-5	1	8938.761	2	NC	1
145		max	.001	1	.003	2	0	1	1.46e-3	4	NC	1	NC	1
146		min	-.002	3	-.003	3	-.002	5	-3.886e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.481e-3	4	NC	1	NC	1
148		min	-.001	3	-.002	3	-.002	5	-4.219e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.502e-3	4	NC	1	NC	1
150		min	0	3	-.001	3	0	5	-4.552e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.523e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-4.884e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.229e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.998e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.004	4	1.964e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-6.894e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.007	4	1.698e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-6.79e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.011	4	1.433e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-6.686e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.015	4	1.168e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-6.582e-4	4	NC	1	NC	1
163		6	max	0	3	.005	2	.019	4	1.048e-5	3	NC	1	NC	1
164			min	-.001	2	-.007	3	0	1	-6.478e-4	4	8532.632	2	NC	1
165		7	max	0	3	.007	2	.022	4	2.661e-5	3	NC	3	NC	1
166			min	-.001	2	-.009	3	0	1	-6.374e-4	4	7075.829	2	NC	1
167		8	max	.001	3	.008	2	.026	4	4.275e-5	3	NC	3	NC	1
168			min	-.001	2	-.01	3	0	1	-6.27e-4	4	6002.4	2	NC	1
169		9	max	.001	3	.009	2	.029	4	5.889e-5	3	NC	3	NC	1
170			min	-.002	2	-.011	3	0	1	-6.166e-4	4	5173.825	2	NC	1
171		10	max	.001	3	.01	2	.033	4	7.503e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	1	-6.062e-4	4	4513.351	2	NC	1
173		11	max	.002	3	.012	2	.036	4	9.116e-5	3	NC	3	NC	1
174			min	-.002	2	-.014	3	0	1	-5.958e-4	4	3974.805	2	NC	1
175		12	max	.002	3	.013	2	.039	4	1.073e-4	3	NC	3	NC	1
176			min	-.002	2	-.015	3	0	1	-5.854e-4	4	3528.496	2	NC	1
177		13	max	.002	3	.015	2	.042	4	1.234e-4	3	NC	3	NC	1
178			min	-.002	2	-.016	3	0	1	-5.75e-4	4	3154.245	2	NC	1
179		14	max	.002	3	.016	2	.045	4	1.396e-4	3	NC	3	NC	1
180			min	-.003	2	-.017	3	0	1	-5.646e-4	4	2837.684	2	NC	1
181		15	max	.002	3	.018	2	.048	4	1.557e-4	3	NC	3	NC	1
182			min	-.003	2	-.017	3	0	1	-5.542e-4	4	2568.189	2	NC	1
183		16	max	.002	3	.02	2	.051	4	1.719e-4	3	NC	3	NC	1
184			min	-.003	2	-.018	3	-.001	1	-5.438e-4	4	2337.662	2	NC	1
185		17	max	.002	3	.022	2	.054	4	1.88e-4	3	NC	3	NC	1
186			min	-.003	2	-.019	3	-.001	1	-5.334e-4	4	2139.786	2	NC	1
187		18	max	.003	3	.023	2	.057	4	2.041e-4	3	NC	3	NC	1
188			min	-.003	2	-.019	3	-.001	1	-5.23e-4	4	1969.542	2	NC	1
189		19	max	.003	3	.025	2	.059	4	2.203e-4	3	NC	3	NC	1
190			min	-.004	2	-.02	3	-.001	1	-5.126e-4	4	1822.901	2	NC	1
191	M8	1	max	.005	1	.024	2	.001	1	2.723e-3	4	NC	1	NC	1
192			min	-.001	3	-.018	3	-.062	4	-1.689e-4	3	NC	1	310.83	4
193		2	max	.005	1	.023	2	0	1	2.723e-3	4	NC	1	NC	1
194			min	-.001	3	-.017	3	-.057	4	-1.689e-4	3	NC	1	338.823	4
195		3	max	.005	1	.021	2	0	1	2.723e-3	4	NC	1	NC	1
196			min	-.001	3	-.016	3	-.052	4	-1.689e-4	3	NC	1	372.14	4
197		4	max	.004	1	.02	2	0	1	2.723e-3	4	NC	1	NC	1
198			min	-.001	3	-.015	3	-.047	4	-1.689e-4	3	NC	1	412.18	4
199		5	max	.004	1	.019	2	0	1	2.723e-3	4	NC	1	NC	1
200			min	0	3	-.014	3	-.042	4	-1.689e-4	3	NC	1	460.854	4
201		6	max	.004	1	.017	2	0	1	2.723e-3	4	NC	1	NC	1
202			min	0	3	-.013	3	-.037	4	-1.689e-4	3	NC	1	520.817	4
203		7	max	.003	1	.016	2	0	1	2.723e-3	4	NC	1	NC	1
204			min	0	3	-.012	3	-.032	4	-1.689e-4	3	NC	1	595.85	4
205		8	max	.003	1	.015	2	0	1	2.723e-3	4	NC	1	NC	1
206			min	0	3	-.011	3	-.028	4	-1.689e-4	3	NC	1	691.491	4
207		9	max	.003	1	.013	2	0	1	2.723e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.024	4	-1.689e-4	3	NC	1	816.115	4
209		10	max	.003	1	.012	2	0	1	2.723e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.02	4	-1.689e-4	3	NC	1	982.886	4
211		11	max	.002	1	.011	2	0	1	2.723e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.016	4	-1.689e-4	3	NC	1	1213.486	4
213		12	max	.002	1	.009	2	0	1	2.723e-3	4	NC	1	NC	1

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.007	3	-.013	4	-1.689e-4	3	NC	1	1545.731	4
215		13	max	.002	1	.008	2	0	1	2.723e-3	4	NC	1	NC	1
216			min	0	3	-.006	3	-.009	4	-1.689e-4	3	NC	1	2050.555	4
217		14	max	.001	1	.007	2	0	1	2.723e-3	4	NC	1	NC	1
218			min	0	3	-.005	3	-.007	4	-1.689e-4	3	NC	1	2874.265	4
219		15	max	.001	1	.005	2	0	1	2.723e-3	4	NC	1	NC	1
220			min	0	3	-.004	3	-.004	4	-1.689e-4	3	NC	1	4360.579	4
221		16	max	0	1	.004	2	0	1	2.723e-3	4	NC	1	NC	1
222			min	0	3	-.003	3	-.003	4	-1.689e-4	3	NC	1	7486.574	4
223		17	max	0	1	.003	2	0	1	2.723e-3	4	NC	1	NC	1
224			min	0	3	-.002	3	-.001	4	-1.689e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	2.723e-3	4	NC	1	NC	1
226			min	0	3	0	3	0	4	-1.689e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	2.723e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.689e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.007	2	0	3	3.983e-4	1	NC	3	NC	1
230			min	-.003	3	-.007	3	-.005	4	-3.559e-4	3	4457.28	2	NC	1
231		2	max	.002	1	.007	2	0	3	3.783e-4	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.005	4	-3.45e-4	3	4834.887	2	NC	1
233		3	max	.002	1	.006	2	0	3	3.582e-4	1	NC	3	NC	1
234			min	-.002	3	-.006	3	-.005	4	-3.341e-4	3	5278.913	2	NC	1
235		4	max	.002	1	.006	2	0	3	3.639e-4	4	NC	3	NC	1
236			min	-.002	3	-.006	3	-.005	4	-3.232e-4	3	5804.559	2	NC	1
237		5	max	.002	1	.005	2	0	3	4.16e-4	4	NC	1	NC	1
238			min	-.002	3	-.006	3	-.005	4	-3.124e-4	3	6431.85	2	NC	1
239		6	max	.002	1	.005	2	0	3	4.681e-4	4	NC	1	NC	1
240			min	-.002	3	-.005	3	-.005	4	-3.015e-4	3	7187.601	2	NC	1
241		7	max	.001	1	.004	2	0	3	5.203e-4	4	NC	1	NC	1
242			min	-.002	3	-.005	3	-.005	4	-2.906e-4	3	8108.387	2	NC	1
243		8	max	.001	1	.004	2	0	3	5.724e-4	4	NC	1	NC	1
244			min	-.002	3	-.005	3	-.004	4	-2.797e-4	3	9245.195	2	NC	1
245		9	max	.001	1	.003	2	0	3	6.245e-4	4	NC	1	NC	1
246			min	-.001	3	-.004	3	-.004	4	-2.688e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	6.766e-4	4	NC	1	NC	1
248			min	-.001	3	-.004	3	-.004	4	-2.579e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	7.288e-4	4	NC	1	NC	1
250			min	-.001	3	-.004	3	-.004	4	-2.47e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	7.809e-4	4	NC	1	NC	1
252			min	-.001	3	-.003	3	-.003	4	-2.361e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	8.33e-4	4	NC	1	NC	1
254			min	0	3	-.003	3	-.003	4	-2.253e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	8.852e-4	4	NC	1	NC	1
256			min	0	3	-.003	3	-.003	4	-2.144e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	9.373e-4	4	NC	1	NC	1
258			min	0	3	-.002	3	-.002	4	-2.035e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	9.894e-4	4	NC	1	NC	1
260			min	0	3	-.002	3	-.002	4	-1.926e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.042e-3	4	NC	1	NC	1
262			min	0	3	-.001	3	-.001	4	-1.817e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.094e-3	4	NC	1	NC	1
264			min	0	3	0	3	0	4	-1.708e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.146e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.599e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.367e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-5.273e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.003	4	5.731e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-5.805e-4	4	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	4.095e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.337e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.459e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-6.869e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.011	4	8.233e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-7.401e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.014	4	-4.913e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-7.933e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.017	5	-5.597e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-8.465e-4	4	NC	1	NC	1
281		8	max	0	3	.001	2	.02	5	-6.281e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-8.998e-4	4	NC	1	NC	1
283		9	max	0	3	.002	2	.023	5	-6.964e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-9.53e-4	4	NC	1	NC	1
285		10	max	0	3	.002	2	.026	5	-7.648e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-1.006e-3	4	NC	1	NC	1
287		11	max	0	3	.002	2	.029	5	-8.332e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.002	3	-1.059e-3	4	NC	1	NC	1
289		12	max	0	3	.003	2	.032	5	-9.016e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.002	1	-1.113e-3	4	NC	1	NC	1
291		13	max	0	3	.004	2	.035	5	-9.7e-6	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	1	-1.166e-3	4	NC	1	NC	1
293		14	max	0	3	.005	2	.038	5	-1.038e-5	10	NC	1	NC	1
294			min	0	2	-.008	3	-.003	1	-1.219e-3	4	NC	1	NC	1
295		15	max	0	3	.005	2	.041	5	-1.107e-5	10	NC	1	NC	1
296			min	0	2	-.008	3	-.003	1	-1.272e-3	4	8586.409	2	NC	1
297		16	max	0	3	.006	2	.043	5	-1.175e-5	10	NC	1	NC	1
298			min	0	2	-.008	3	-.004	1	-1.325e-3	4	7326.004	2	NC	1
299		17	max	0	3	.007	2	.046	5	-1.244e-5	10	NC	3	NC	1
300			min	0	2	-.008	3	-.004	1	-1.379e-3	4	6339.071	2	NC	1
301		18	max	0	3	.008	2	.049	5	-1.312e-5	10	NC	3	NC	1
302			min	0	2	-.008	3	-.004	1	-1.432e-3	4	5559.364	2	NC	1
303		19	max	0	3	.009	2	.052	5	-1.38e-5	10	NC	3	NC	2
304			min	0	2	-.008	3	-.005	1	-1.485e-3	4	4939.053	2	9560.211	1
305	M12	1	max	.002	1	.008	2	.004	1	3.551e-3	4	NC	1	NC	2
306			min	0	3	-.007	3	-.057	5	1.39e-5	10	NC	1	338.31	5
307		2	max	.002	1	.008	2	.004	1	3.551e-3	4	NC	1	NC	2
308			min	0	3	-.007	3	-.052	5	1.39e-5	10	NC	1	368.771	5
309		3	max	.002	1	.008	2	.003	1	3.551e-3	4	NC	1	NC	2
310			min	0	3	-.006	3	-.048	5	1.39e-5	10	NC	1	405.023	5
311		4	max	.001	1	.007	2	.003	1	3.551e-3	4	NC	1	NC	2
312			min	0	3	-.006	3	-.043	5	1.39e-5	10	NC	1	448.59	5
313		5	max	.001	1	.007	2	.003	1	3.551e-3	4	NC	1	NC	2
314			min	0	3	-.005	3	-.039	5	1.39e-5	10	NC	1	501.549	5
315		6	max	.001	1	.006	2	.002	1	3.551e-3	4	NC	1	NC	2
316			min	0	3	-.005	3	-.034	5	1.39e-5	10	NC	1	566.79	5
317		7	max	.001	1	.006	2	.002	1	3.551e-3	4	NC	1	NC	2
318			min	0	3	-.005	3	-.03	5	1.39e-5	10	NC	1	648.427	5
319		8	max	.001	1	.005	2	.002	1	3.551e-3	4	NC	1	NC	1
320			min	0	3	-.004	3	-.026	5	1.39e-5	10	NC	1	752.483	5
321		9	max	0	1	.005	2	.002	1	3.551e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.022	5	1.39e-5	10	NC	1	888.07	5
323		10	max	0	1	.004	2	.001	1	3.551e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.018	5	1.39e-5	10	NC	1	1069.507	5
325		11	max	0	1	.004	2	.001	1	3.551e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.015	5	1.39e-5	10	NC	1	1320.382	5
327		12	max	0	1	.003	2	0	1	3.551e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.011	5	1.39e-5	10	NC	1	1681.832	5
329		max	0	1	.003	2	0	1	3.551e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.009	5	1.39e-5	10	NC	1	2231.019	5
331		max	0	1	.002	2	0	1	3.551e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.006	5	1.39e-5	10	NC	1	3127.1	5
333		max	0	1	.002	2	0	1	3.551e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.004	5	1.39e-5	10	NC	1	4743.966	5
335		max	0	1	.001	2	0	1	3.551e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.002	5	1.39e-5	10	NC	1	8144.467	5
337		max	0	1	0	2	0	1	3.551e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.001	5	1.39e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	3.551e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	1.39e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.551e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.39e-5	10	NC	1	NC	1
343	M1	max	.006	3	.024	3	.006	5	5.394e-3	1	NC	1	NC	1
344		min	-.007	2	-.022	1	-.002	1	-6.556e-3	3	NC	1	NC	1
345		max	.006	3	.013	3	.008	5	2.56e-3	1	NC	4	NC	1
346		min	-.007	2	-.012	1	-.004	1	-3.222e-3	3	4505.164	3	NC	1
347		max	.006	3	.003	3	.011	5	2.791e-4	5	NC	4	NC	1
348		min	-.007	2	-.002	1	-.005	1	-2.212e-4	1	2327.629	2	9730.898	5
349		max	.006	3	.006	2	.014	5	2.767e-4	5	NC	4	NC	1
350		min	-.007	2	-.005	3	-.006	1	-1.843e-4	1	1633.488	2	6077.991	5
351		max	.006	3	.013	2	.017	5	2.743e-4	5	NC	5	NC	1
352		min	-.007	2	-.012	3	-.006	1	-1.475e-4	1	1297.894	2	4313.154	5
353		max	.006	3	.019	2	.02	5	2.719e-4	5	NC	5	NC	1
354		min	-.007	2	-.017	3	-.005	1	-1.107e-4	1	1106.573	2	3291.498	5
355		max	.006	3	.024	2	.024	5	2.696e-4	5	NC	5	NC	1
356		min	-.007	2	-.021	3	-.005	1	-7.392e-5	1	989.051	2	2634.97	5
357		max	.006	3	.027	2	.028	5	2.672e-4	5	NC	5	NC	1
358		min	-.007	2	-.024	3	-.004	1	-3.711e-5	1	915.902	2	2183.13	5
359		max	.006	3	.029	2	.031	5	2.648e-4	5	NC	5	NC	1
360		min	-.007	2	-.026	3	-.003	1	-7.781e-6	9	873.262	2	1854.464	4
361		max	.006	3	.03	2	.035	5	2.704e-4	4	NC	5	NC	1
362		min	-.007	2	-.026	3	-.001	1	4.296e-6	10	854.563	2	1595.083	4
363		max	.006	3	.03	2	.039	4	2.761e-4	4	NC	5	NC	1
364		min	-.007	2	-.025	3	0	9	5.785e-6	10	857.514	2	1398.758	4
365		max	.006	3	.028	2	.043	4	2.818e-4	4	NC	5	NC	1
366		min	-.007	2	-.023	3	0	10	7.274e-6	10	883.23	2	1246.866	4
367		max	.006	3	.025	2	.047	4	2.875e-4	4	NC	5	NC	1
368		min	-.007	2	-.02	3	0	10	8.763e-6	10	936.79	2	1127.387	4
369		max	.006	3	.02	2	.051	4	2.932e-4	4	NC	5	NC	1
370		min	-.007	2	-.016	3	0	10	1.025e-5	10	1029.758	2	1032.275	4
371		max	.006	3	.013	2	.055	4	2.989e-4	4	NC	4	NC	1
372		min	-.007	2	-.011	3	0	10	1.174e-5	10	1187.347	2	955.991	4
373		max	.006	3	.005	2	.058	4	4.978e-4	4	NC	4	NC	1
374		min	-.007	2	-.004	3	0	10	1.288e-5	10	1470.686	2	894.629	4
375		max	.006	3	.003	3	.061	4	5.295e-3	4	NC	4	NC	1
376		min	-.007	2	-.004	2	0	10	5.57e-6	10	2071.004	2	845.451	4
377		max	.006	3	.01	3	.064	4	3.377e-3	2	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-1.597e-3	3	3992.42	2	806.324	4
379		max	.006	3	.018	3	.066	4	6.78e-3	2	NC	1	NC	1
380		min	-.007	2	-.027	2	-.001	1	-3.264e-3	3	NC	1	776.66	4
381	M5	max	.017	3	.065	3	.006	5	9.101e-6	4	NC	1	NC	1
382		min	-.021	2	-.06	1	-.002	1	4.355e-8	11	NC	1	NC	1
383		max	.017	3	.036	3	.008	5	1.355e-4	5	NC	4	NC	1
384		min	-.021	2	-.033	1	-.002	1	-3.7e-5	1	1696.681	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.017	3	.01	3	.011	5	2.599e-4	5	NC	5	NC	1
386			min	-.021	2	-.007	1	-.002	1	-7.331e-5	1	872.06	1	NC	1
387		4	max	.017	3	.015	2	.014	5	2.711e-4	5	NC	5	NC	1
388			min	-.021	2	-.012	3	-.002	1	-6.922e-5	1	612.121	2	NC	1
389		5	max	.017	3	.034	2	.017	5	2.823e-4	5	NC	5	NC	1
390			min	-.021	2	-.03	3	-.002	1	-6.513e-5	1	485.398	2	NC	1
391		6	max	.017	3	.05	2	.021	5	2.935e-4	5	NC	5	NC	1
392			min	-.021	2	-.045	3	-.002	1	-6.105e-5	1	413.119	2	NC	1
393		7	max	.017	3	.063	2	.025	5	3.047e-4	5	NC	5	NC	1
394			min	-.021	2	-.056	3	-.002	1	-5.696e-5	1	368.654	2	NC	1
395		8	max	.017	3	.073	2	.029	5	3.159e-4	5	NC	5	NC	1
396			min	-.021	2	-.063	3	-.002	1	-5.288e-5	1	340.887	2	NC	1
397		9	max	.017	3	.079	2	.033	5	3.272e-4	5	NC	5	NC	1
398			min	-.021	2	-.067	3	-.001	1	-4.879e-5	1	324.579	2	NC	1
399		10	max	.016	3	.081	2	.037	5	3.384e-4	5	NC	5	NC	1
400			min	-.021	2	-.067	3	-.001	1	-4.471e-5	1	317.237	2	NC	1
401		11	max	.016	3	.08	2	.041	5	3.496e-4	5	NC	5	NC	1
402			min	-.021	2	-.065	3	-.001	1	-4.062e-5	1	317.978	2	NC	1
403		12	max	.016	3	.075	2	.045	4	3.608e-4	5	NC	5	NC	1
404			min	-.021	2	-.06	3	-.001	1	-3.653e-5	1	327.194	2	NC	1
405		13	max	.016	3	.066	2	.049	4	3.72e-4	5	NC	5	NC	1
406			min	-.021	2	-.051	3	-.001	1	-3.245e-5	1	346.753	2	NC	1
407		14	max	.016	3	.053	2	.053	4	3.832e-4	5	NC	5	NC	1
408			min	-.021	2	-.041	3	-.001	1	-2.836e-5	1	380.936	2	NC	1
409		15	max	.016	3	.035	2	.056	4	3.944e-4	5	NC	5	NC	1
410			min	-.021	2	-.027	3	-.001	1	-2.428e-5	1	439.113	2	NC	1
411		16	max	.016	3	.014	2	.059	4	5.952e-4	4	NC	5	NC	1
412			min	-.021	2	-.011	3	-.001	1	-2.259e-5	1	544.085	2	NC	1
413		17	max	.016	3	.007	3	.062	4	5.325e-3	4	NC	5	NC	1
414			min	-.021	2	-.012	2	-.001	1	-7.801e-5	1	767.868	2	NC	1
415		18	max	.016	3	.027	3	.064	4	2.733e-3	4	NC	4	NC	1
416			min	-.021	2	-.043	2	0	1	-3.992e-5	1	1489.375	2	NC	1
417		19	max	.016	3	.048	3	.066	4	3.515e-6	5	NC	1	NC	1
418			min	-.021	2	-.075	2	0	1	-3.703e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.023	3	.005	5	6.561e-3	3	NC	1	NC	1
420			min	-.007	2	-.022	1	-.002	1	-5.394e-3	1	NC	1	NC	1
421		2	max	.006	3	.013	3	.005	5	3.252e-3	3	NC	4	NC	1
422			min	-.007	2	-.012	1	0	9	-2.64e-3	1	4506.441	3	NC	1
423		3	max	.006	3	.003	3	.005	4	6.228e-5	1	NC	4	NC	1
424			min	-.007	2	-.002	1	0	3	-2.153e-5	5	2328.027	2	NC	1
425		4	max	.006	3	.006	2	.006	4	3.557e-5	2	NC	4	NC	1
426			min	-.007	2	-.005	3	-.001	3	-2.922e-5	5	1633.783	2	NC	1
427		5	max	.006	3	.013	2	.007	4	2.484e-5	2	NC	4	NC	1
428			min	-.007	2	-.012	3	-.002	3	-3.823e-5	4	1298.141	2	NC	1
429		6	max	.006	3	.019	2	.009	4	1.412e-5	2	NC	5	NC	1
430			min	-.007	2	-.017	3	-.002	3	-5.19e-5	4	1106.794	2	9814.471	4
431		7	max	.006	3	.024	2	.012	4	3.394e-6	2	NC	5	NC	1
432			min	-.007	2	-.022	3	-.003	3	-6.558e-5	4	989.258	2	6188.262	4
433		8	max	.006	3	.027	2	.015	4	-1.189e-6	10	NC	5	NC	1
434			min	-.007	2	-.024	3	-.003	3	-8.745e-5	1	916.102	2	4315.651	4
435		9	max	.006	3	.029	2	.019	4	-2.686e-6	10	NC	5	NC	1
436			min	-.007	2	-.026	3	-.003	3	-1.174e-4	1	873.46	2	3216.829	4
437		10	max	.006	3	.03	2	.023	5	-4.183e-6	10	NC	5	NC	1
438			min	-.007	2	-.026	3	-.003	3	-1.473e-4	1	854.764	2	2514.559	4
439		11	max	.006	3	.03	2	.028	5	-5.679e-6	10	NC	5	NC	1
440			min	-.007	2	-.025	3	-.003	3	-1.773e-4	1	857.723	2	2037.385	4
441		12	max	.006	3	.028	2	.033	5	-7.176e-6	10	NC	5	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.006	4	-0.02	1	-0.032	3	-4.888e-3	1	3603.302	1	1551.364	3
557	13	max	0	3	.003	5	.03	1	5.443e-3	3	NC	5	7034.048	15
558		min	-0.006	4	-0.019	1	-0.032	3	-5.329e-3	1	3902.187	1	1534.583	3
559	14	max	0	3	.003	5	.028	1	5.868e-3	3	NC	5	NC	15
560		min	-0.007	4	-0.017	1	-0.029	3	-5.771e-3	1	4400.203	1	1581.178	3
561	15	max	0	3	.003	5	.024	1	6.293e-3	3	NC	5	NC	5
562		min	-0.008	4	-0.014	1	-0.025	3	-6.212e-3	1	5228.343	1	1715.534	3
563	16	max	0	3	.004	5	.018	1	6.717e-3	3	NC	5	NC	4
564		min	-0.008	4	-0.012	1	-0.018	3	-6.654e-3	1	6700.34	1	2004.108	3
565	17	max	.001	3	.004	5	.009	1	7.142e-3	3	NC	3	NC	4
566		min	-0.009	4	-0.009	1	-0.009	3	-7.096e-3	1	9766.427	1	2655.601	3
567	18	max	.001	3	.004	5	.004	3	7.567e-3	3	NC	1	NC	4
568		min	-0.009	4	-0.005	1	-0.007	2	-7.537e-3	1	NC	1	4725.986	3
569	19	max	.001	3	.005	5	.019	3	7.992e-3	3	NC	1	NC	1
570		min	-.01	4	-0.002	9	-0.022	2	-7.979e-3	1	NC	1	NC	1
571	M16A	1	max	0	10	0	.007	3	2.848e-3	3	NC	1	NC	1
572		min	-0.003	4	-0.003	4	-0.008	2	-2.894e-3	2	NC	1	NC	1
573	2	max	0	10	-0.002	12	.002	9	2.723e-3	3	NC	1	NC	1
574		min	-0.003	4	-0.009	4	-0.002	2	-2.755e-3	2	NC	1	NC	1
575	3	max	0	10	-0.003	12	.006	1	2.597e-3	3	NC	3	NC	4
576		min	-0.003	4	-0.015	4	-0.005	5	-2.616e-3	2	5640.41	4	6342.232	1
577	4	max	0	10	-0.005	12	.01	1	2.472e-3	3	NC	12	NC	4
578		min	-0.003	4	-0.02	4	-0.009	5	-2.478e-3	2	3869.651	4	4814.988	1
579	5	max	0	10	-0.006	12	.012	1	2.347e-3	3	NC	12	NC	10
580		min	-0.003	4	-0.025	4	-0.014	5	-2.339e-3	2	3019.528	4	4149.785	1
581	6	max	0	10	-0.008	12	.014	1	2.221e-3	3	8800.407	12	NC	14
582		min	-0.002	4	-0.029	4	-0.019	5	-2.201e-3	2	2541.252	4	3719.548	5
583	7	max	0	10	-0.009	12	.015	1	2.096e-3	3	7804.373	12	NC	14
584		min	-0.002	4	-0.032	4	-0.024	5	-2.062e-3	2	2253.632	4	2927.981	5
585	8	max	0	10	-0.009	12	.014	1	1.97e-3	3	7206.603	12	9524.659	10
586		min	-0.002	4	-0.035	4	-0.028	5	-1.924e-3	2	2081.017	4	2473.353	5
587	9	max	0	10	-0.01	12	.014	1	1.845e-3	3	6884.848	12	NC	10
588		min	-0.002	4	-0.036	4	-0.032	5	-1.785e-3	2	1988.105	4	2209.277	5
589	10	max	0	10	-0.01	12	.013	1	1.72e-3	3	6783.06	12	NC	10
590		min	-0.002	4	-0.036	4	-0.034	5	-1.647e-3	2	1958.712	4	2069.665	5
591	11	max	0	10	-0.01	12	.011	1	1.594e-3	3	6884.848	12	NC	9
592		min	-0.002	4	-0.036	4	-0.034	5	-1.508e-3	2	1988.105	4	2025.248	5
593	12	max	0	10	-0.009	12	.009	1	1.469e-3	3	7206.603	12	NC	9
594		min	-0.001	4	-0.034	4	-0.034	5	-1.37e-3	2	2081.017	4	2068.318	5
595	13	max	0	10	-0.009	12	.007	1	1.343e-3	3	7804.373	12	NC	9
596		min	-0.001	4	-0.031	4	-0.031	5	-1.231e-3	2	2253.632	4	2209.538	5
597	14	max	0	10	-0.008	12	.005	1	1.218e-3	3	8800.407	12	NC	1
598		min	0	4	-0.028	4	-0.028	5	-1.092e-3	2	2541.252	4	2484.334	5
599	15	max	0	10	-0.007	12	.003	1	1.093e-3	3	NC	12	NC	1
600		min	0	4	-0.023	4	-0.023	5	-9.539e-4	2	3019.528	4	2976.516	5
601	16	max	0	10	-0.005	12	.002	1	9.672e-4	3	NC	12	NC	1
602		min	0	4	-0.018	4	-0.018	5	-8.154e-4	2	3869.651	4	3894.569	5
603	17	max	0	10	-0.004	12	0	9	8.418e-4	3	NC	3	NC	1
604		min	0	4	-0.012	4	-0.012	5	-6.768e-4	2	5640.41	4	5882.377	5
605	18	max	0	10	-0.002	12	0	9	7.786e-4	4	NC	1	NC	1
606		min	0	4	-0.006	4	-0.006	5	-5.383e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	8.459e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-3.997e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
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Address:			
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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.