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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	160 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 40.19 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

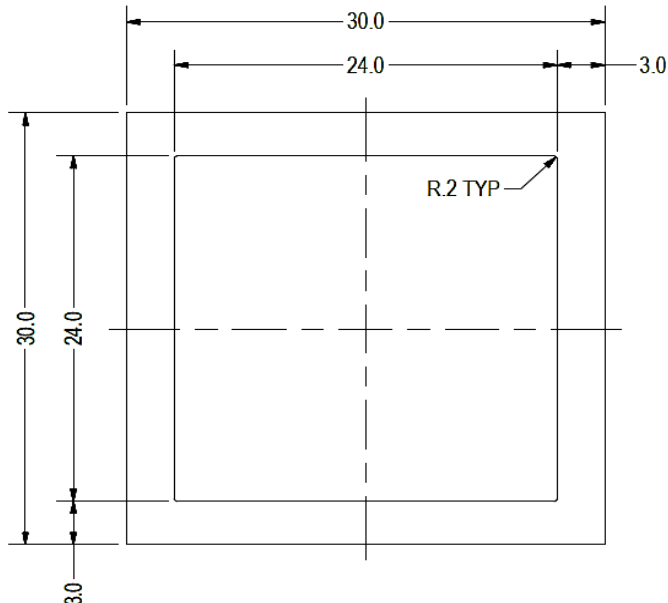
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.062 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>9%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.188 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>5%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

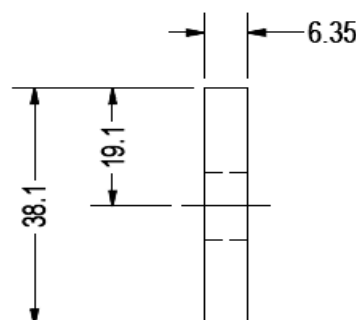
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.794 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>10%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.070 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>5%</b>



A cross brace kit is required every 40 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

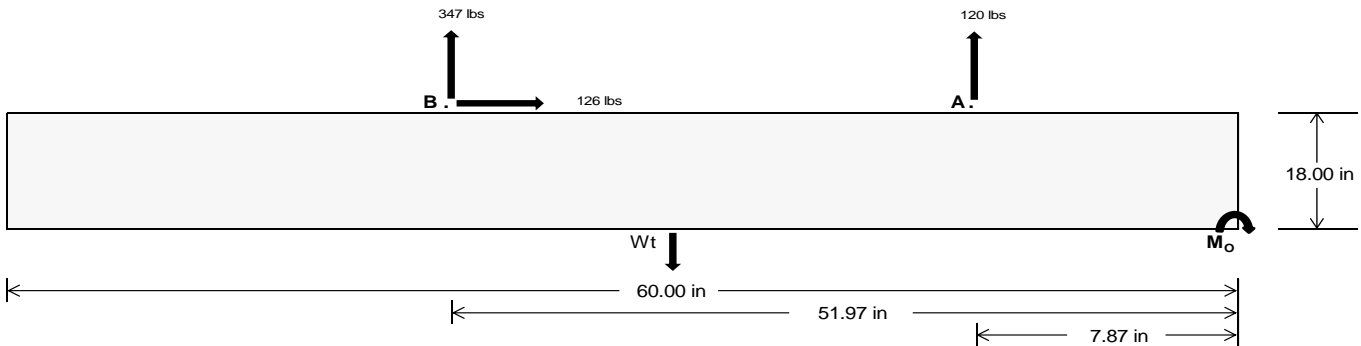
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>523.21</b>	<b>1508.40</b>	k
Compressive Load =	<b>1380.22</b>	<b>985.57</b>	k
Lateral Load =	<b>1.44</b>	<b>547.66</b>	k
Moment (Weak Axis) =	<b>0.00</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21265.0$  in-lbs  
Resisting Force Required = 708.83 lbs  
S.F. = 1.67  
Weight Required = 1181.39 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 126.37 lbs  
Friction = 0.4  
Weight Required = 315.94 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 126.37 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	419 lbs	419 lbs	419 lbs	419 lbs	546 lbs	546 lbs	546 lbs	546 lbs	694 lbs	694 lbs	694 lbs	694 lbs	-239 lbs	-239 lbs	-239 lbs	-239 lbs
$F_B$	302 lbs	302 lbs	302 lbs	302 lbs	390 lbs	390 lbs	390 lbs	390 lbs	497 lbs	497 lbs	497 lbs	497 lbs	-695 lbs	-695 lbs	-695 lbs	-695 lbs
$F_V$	21 lbs	21 lbs	21 lbs	21 lbs	219 lbs	219 lbs	219 lbs	219 lbs	179 lbs	179 lbs	179 lbs	179 lbs	-253 lbs	-253 lbs	-253 lbs	-253 lbs
$P_{total}$	2624 lbs	2715 lbs	2805 lbs	2896 lbs	2839 lbs	2929 lbs	3020 lbs	3111 lbs	3094 lbs	3184 lbs	3275 lbs	3366 lbs	208 lbs	262 lbs	317 lbs	371 lbs
$M$	252 lbs-ft	252 lbs-ft	252 lbs-ft	252 lbs-ft	621 lbs-ft	621 lbs-ft	621 lbs-ft	621 lbs-ft	639 lbs-ft	639 lbs-ft	639 lbs-ft	639 lbs-ft	451 lbs-ft	451 lbs-ft	451 lbs-ft	451 lbs-ft
$e$	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.20 ft	0.19 ft	2.17 ft	1.72 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	265.3 psf	263.2 psf	261.2 psf	259.4 psf	239.2 psf	238.2 psf	237.3 psf	236.5 psf	265.9 psf	263.7 psf	261.7 psf	259.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	334.4 psf	329.1 psf	324.2 psf	319.8 psf	409.6 psf	400.9 psf	392.9 psf	385.6 psf	441.2 psf	431.0 psf	421.8 psf	413.3 psf	239.9 psf	122.4 psf	102.5 psf	96.4 psf

Maximum Bearing Pressure = 441 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

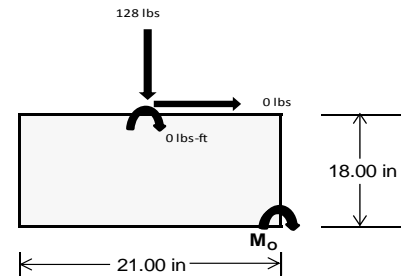
#### Overturning Check

$M_o = 0.0 \text{ ft-lbs}$   
 Resisting Force Required = 0.00 lbs  
 S.F. = 1.67  
 Weight Required = 0.00 lbs  
 Minimum Width = **21 in**  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	50 lbs	128 lbs	48 lbs	196 lbs	585 lbs	193 lbs	15 lbs	37 lbs	14 lbs
$F_v$	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2406 lbs	2484 lbs	2404 lbs	2439 lbs	2828 lbs	2436 lbs	704 lbs	726 lbs	703 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
$f_{min}$	275.0 sqft	283.9 sqft	274.7 sqft	278.5 sqft	323.1 sqft	278.3 sqft	80.4 sqft	83.0 sqft	80.3 sqft
$f_{max}$	275.0 psf	283.9 psf	274.7 psf	278.9 psf	323.3 psf	278.5 psf	80.4 psf	83.0 psf	80.3 psf



Maximum Bearing Pressure = 323 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



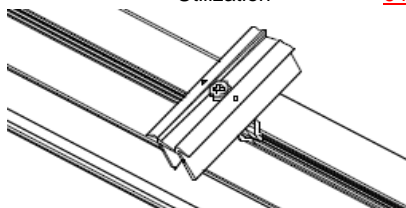
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

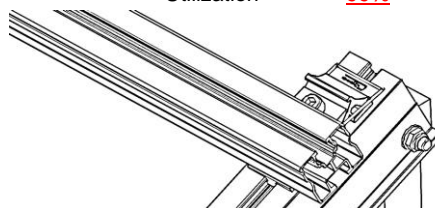
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.773 k
Allowable Uplift =	1.214 k
Utilization =	<u>64%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.101 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

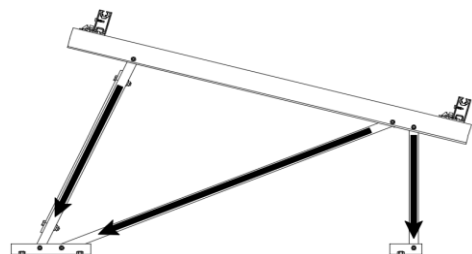
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.062 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Diagonal Strut

Maximum Axial Load =	0.188 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.142 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.070 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.003 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$124.989$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$129.794$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.7$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.266 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

### 3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.41$$

$$20.702$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

### 3.4.15

N/A for Strong Direction

### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.16

N/A for Strong Direction

Weak Axis:

### 3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.41$$

$$24.5845$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

### 3.4.16

N/A for Weak Direction

### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b [Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.466 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.248	-63.248	0	0
2	M16	Y	-63.248	-63.248	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-112.091	-112.091	0	0
2	M16	y	-179.345	-179.345	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	228.665	228.665	0	0
2	M16	y	112.091	112.091	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								
8																				
9	ASD 1.0D + 1.0S	Yes	Y		1	1	3	1												
10	ASD 1.0D + 0.6W	Yes	Y		1	1			4	.6										
11	ASD 1.0D + 0.75L + 0.45W + 0....	Yes	Y		1	1	3	.75	4	.45										
12	ASD 0.6D + 0.6W	Yes	Y		2	.6					5	.6								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y		1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y		1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y		1	.362					6	.875								



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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	115.378	2	240.1	2	.004	10	0	9	0	1	0	1
2		min	-141.796	3	-363.676	3	-.192	3	0	3	0	1	0	1
3	N7	max	0	15	348.883	1	.022	10	0	10	0	1	0	1
4		min	-.108	2	-118.369	3	-.36	1	0	1	0	1	0	1
5	N15	max	0	15	1061.707	2	.085	9	0	9	0	1	0	1
6		min	-1.105	2	-402.471	3	-.454	3	0	3	0	1	0	1
7	N16	max	373.999	2	758.131	2	0	11	0	9	0	1	0	1
8		min	-421.28	3	-1160.305	3	-58.654	3	0	3	0	1	0	1
9	N23	max	0	15	349.092	1	.477	1	0	1	0	1	0	1
10		min	-.108	2	-.118	3	-.021	10	0	10	0	1	0	1
11	N24	max	115.378	2	242.396	2	59.164	3	0	9	0	1	0	1
12		min	-142.106	3	-362.666	3	-.004	10	0	3	0	1	0	1
13	Totals:	max	603.433	2	2976.777	2	0	2						
14		min	-705.553	3	-2525.486	3	0	3						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	250.919	1	.669	4	.121	1	0	10	0	3	0	1
2			min	-364.982	3	.158	15	-.135	3	0	1	0	2	0	1
3		2	max	251.015	1	.631	4	.121	1	0	10	0	9	0	15
4			min	-364.91	3	.149	15	-.135	3	0	1	0	10	0	4
5		3	max	251.112	1	.593	4	.121	1	0	10	0	1	0	15
6			min	-364.837	3	.14	15	-.135	3	0	1	0	3	0	4
7		4	max	251.208	1	.555	4	.121	1	0	10	0	1	0	15
8			min	-364.765	3	.131	15	-.135	3	0	1	0	3	0	4
9		5	max	251.304	1	.517	4	.121	1	0	10	0	1	0	15
10			min	-364.693	3	.122	15	-.135	3	0	1	0	3	0	4
11		6	max	251.401	1	.48	4	.121	1	0	10	0	1	0	15
12			min	-364.621	3	.114	15	-.135	3	0	1	0	3	0	4
13		7	max	251.497	1	.442	4	.121	1	0	10	0	1	0	15
14			min	-364.548	3	.105	15	-.135	3	0	1	0	3	0	4
15		8	max	251.594	1	.404	4	.121	1	0	10	0	1	0	15
16			min	-364.476	3	.096	15	-.135	3	0	1	0	3	0	4
17		9	max	251.69	1	.366	4	.121	1	0	10	0	1	0	15
18			min	-364.404	3	.087	15	-.135	3	0	1	0	3	0	4
19		10	max	251.786	1	.328	4	.121	1	0	10	0	1	0	15
20			min	-364.332	3	.078	15	-.135	3	0	1	0	3	0	4
21		11	max	251.883	1	.291	4	.121	1	0	10	0	1	0	15
22			min	-364.259	3	.069	15	-.135	3	0	1	0	3	0	4
23		12	max	251.979	1	.253	4	.121	1	0	10	0	1	0	15
24			min	-364.187	3	.06	15	-.135	3	0	1	0	3	0	4
25		13	max	252.075	1	.215	4	.121	1	0	10	0	1	0	15
26			min	-364.115	3	.051	15	-.135	3	0	1	0	3	0	4
27		14	max	252.172	1	.177	4	.121	1	0	10	0	1	0	15
28			min	-364.042	3	.042	15	-.135	3	0	1	0	3	0	4
29		15	max	252.268	1	.139	4	.121	1	0	10	0	1	0	15
30			min	-363.97	3	.034	15	-.135	3	0	1	0	3	0	4
31		16	max	252.364	1	.101	4	.121	1	0	10	0	1	0	15
32			min	-363.898	3	.025	15	-.135	3	0	1	0	3	0	4
33		17	max	252.461	1	.066	2	.121	1	0	10	0	1	0	15
34			min	-363.826	3	.016	15	-.135	3	0	1	0	3	0	4
35		18	max	252.557	1	.037	2	.121	1	0	10	0	1	0	15
36			min	-363.753	3	.002	9	-.135	3	0	1	0	3	0	4
37		19	max	252.654	1	.008	10	.121	1	0	10	0	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-363.681	3	-.023	9	-.135	3	0	1	0	3	0	4
39	M3	1	max	56.05	2	1.817	4	.004	10	0	10	0	1	4
40		min	-48.325	9	.428	15	-.138	1	0	1	0	10	0	15
41		2	max	55.983	2	1.639	4	.004	10	0	10	0	1	4
42		min	-48.38	9	.386	15	-.138	1	0	1	0	10	0	15
43		3	max	55.916	2	1.461	4	.004	10	0	10	0	1	2
44		min	-48.436	9	.344	15	-.138	1	0	1	0	10	0	15
45		4	max	55.848	2	1.283	4	.004	10	0	10	0	1	15
46		min	-48.492	9	.302	15	-.138	1	0	1	0	10	0	4
47		5	max	55.781	2	1.105	4	.004	10	0	10	0	1	15
48		min	-48.548	9	.26	15	-.138	1	0	1	0	10	0	4
49		6	max	55.714	2	.927	4	.004	10	0	10	0	1	15
50		min	-48.604	9	.218	15	-.138	1	0	1	0	10	0	4
51		7	max	55.647	2	.749	4	.004	10	0	10	0	1	15
52		min	-48.66	9	.177	15	-.138	1	0	1	0	10	0	4
53		8	max	55.58	2	.571	4	.004	10	0	10	0	1	15
54		min	-48.716	9	.135	15	-.138	1	0	1	0	10	0	4
55		9	max	55.513	2	.393	4	.004	10	0	10	0	1	15
56		min	-48.772	9	.093	15	-.138	1	0	1	0	10	-.001	4
57		10	max	55.446	2	.215	4	.004	10	0	10	0	1	15
58		min	-48.828	9	.051	15	-.138	1	0	1	0	10	-.001	4
59		11	max	55.379	2	.04	2	.004	10	0	10	0	1	15
60		min	-48.884	9	.009	15	-.138	1	0	1	0	10	-.001	4
61		12	max	55.312	2	-.033	15	.004	10	0	10	0	1	15
62		min	-48.94	9	-.141	4	-.138	1	0	1	0	10	-.001	4
63		13	max	55.245	2	-.075	15	.004	10	0	10	0	1	15
64		min	-48.995	9	-.319	4	-.138	1	0	1	0	10	-.001	4
65		14	max	55.177	2	-.116	15	.004	10	0	10	0	9	15
66		min	-49.051	9	-.497	4	-.138	1	0	1	0	11	-.001	4
67		15	max	55.11	2	-.158	15	.004	10	0	10	0	10	15
68		min	-49.107	9	-.675	4	-.138	1	0	1	0	1	0	4
69		16	max	55.043	2	-.2	15	.004	10	0	10	0	10	15
70		min	-49.163	9	-.853	4	-.138	1	0	1	0	1	0	4
71		17	max	54.976	2	-.242	15	.004	10	0	10	0	10	15
72		min	-49.219	9	-1.031	4	-.138	1	0	1	0	1	0	4
73		18	max	54.909	2	-.284	15	.004	10	0	10	0	10	15
74		min	-49.275	9	-1.209	4	-.138	1	0	1	0	1	0	4
75		19	max	54.842	2	-.326	15	.004	10	0	10	0	10	1
76		min	-49.331	9	-1.387	4	-.138	1	0	1	0	1	0	1
77	M4	1	max	347.718	1	0	1	.022	10	0	1	0	3	1
78		min	-119.242	3	0	1	-.382	1	0	1	0	2	0	1
79		2	max	347.783	1	0	1	.022	10	0	1	0	15	1
80		min	-119.194	3	0	1	-.382	1	0	1	0	1	0	1
81		3	max	347.848	1	0	1	.022	10	0	1	0	15	1
82		min	-119.145	3	0	1	-.382	1	0	1	0	1	0	1
83		4	max	347.912	1	0	1	.022	10	0	1	0	10	1
84		min	-119.097	3	0	1	-.382	1	0	1	0	1	0	1
85		5	max	347.977	1	0	1	.022	10	0	1	0	10	1
86		min	-119.048	3	0	1	-.382	1	0	1	0	1	0	1
87		6	max	348.042	1	0	1	.022	10	0	1	0	10	1
88		min	-119	3	0	1	-.382	1	0	1	0	1	0	1
89		7	max	348.106	1	0	1	.022	10	0	1	0	10	1
90		min	-118.951	3	0	1	-.382	1	0	1	0	1	0	1
91		8	max	348.171	1	0	1	.022	10	0	1	0	10	1
92		min	-118.903	3	0	1	-.382	1	0	1	0	1	0	1
93		9	max	348.236	1	0	1	.022	10	0	1	0	10	1
94		min	-118.854	3	0	1	-.382	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	348.3	1	0	1	.022	10	0	1	0	10	0	1
96		min	-118.806	3	0	1	-.382	1	0	1	0	1	0	1
97	11	max	348.365	1	0	1	.022	10	0	1	0	10	0	1
98		min	-118.757	3	0	1	-.382	1	0	1	0	1	0	1
99	12	max	348.43	1	0	1	.022	10	0	1	0	10	0	1
100		min	-118.709	3	0	1	-.382	1	0	1	0	1	0	1
101	13	max	348.495	1	0	1	.022	10	0	1	0	10	0	1
102		min	-118.66	3	0	1	-.382	1	0	1	0	1	0	1
103	14	max	348.559	1	0	1	.022	10	0	1	0	10	0	1
104		min	-118.612	3	0	1	-.382	1	0	1	0	1	0	1
105	15	max	348.624	1	0	1	.022	10	0	1	0	10	0	1
106		min	-118.563	3	0	1	-.382	1	0	1	0	1	0	1
107	16	max	348.689	1	0	1	.022	10	0	1	0	10	0	1
108		min	-118.515	3	0	1	-.382	1	0	1	0	1	0	1
109	17	max	348.753	1	0	1	.022	10	0	1	0	10	0	1
110		min	-118.466	3	0	1	-.382	1	0	1	0	1	0	1
111	18	max	348.818	1	0	1	.022	10	0	1	0	10	0	1
112		min	-118.417	3	0	1	-.382	1	0	1	0	1	0	1
113	19	max	348.883	1	0	1	.022	10	0	1	0	10	0	1
114		min	-118.369	3	0	1	-.382	1	0	1	0	1	0	1
115	M6	1	max	792.418	1	.658	.031	9	0	3	0	3	0	1
116		min	-1141.887	3	.156	15	-.289	3	0	2	0	1	0	1
117	2	max	792.514	1	.62	4	.031	9	0	3	0	3	0	15
118		min	-1141.815	3	.148	15	-.289	3	0	2	0	1	0	4
119	3	max	792.61	1	.582	4	.031	9	0	3	0	3	0	15
120		min	-1141.743	3	.139	15	-.289	3	0	2	0	2	0	4
121	4	max	792.707	1	.544	4	.031	9	0	3	0	3	0	15
122		min	-1141.671	3	.13	15	-.289	3	0	2	0	2	0	4
123	5	max	792.803	1	.507	4	.031	9	0	3	0	9	0	15
124		min	-1141.598	3	.121	15	-.289	3	0	2	0	3	0	4
125	6	max	792.899	1	.469	4	.031	9	0	3	0	9	0	15
126		min	-1141.526	3	.112	15	-.289	3	0	2	0	3	0	4
127	7	max	792.996	1	.431	4	.031	9	0	3	0	9	0	15
128		min	-1141.454	3	.103	15	-.289	3	0	2	0	3	0	4
129	8	max	793.092	1	.393	4	.031	9	0	3	0	9	0	15
130		min	-1141.381	3	.094	15	-.289	3	0	2	0	3	0	4
131	9	max	793.189	1	.355	4	.031	9	0	3	0	9	0	15
132		min	-1141.309	3	.085	15	-.289	3	0	2	0	3	0	4
133	10	max	793.285	1	.317	4	.031	9	0	3	0	9	0	15
134		min	-1141.237	3	.076	15	-.289	3	0	2	0	3	0	4
135	11	max	793.381	1	.28	4	.031	9	0	3	0	9	0	15
136		min	-1141.165	3	.067	15	-.289	3	0	2	0	3	0	4
137	12	max	793.478	1	.242	4	.031	9	0	3	0	9	0	15
138		min	-1141.092	3	.059	15	-.289	3	0	2	0	3	0	4
139	13	max	793.574	1	.212	2	.031	9	0	3	0	9	0	15
140		min	-1141.02	3	.05	15	-.289	3	0	2	0	3	0	4
141	14	max	793.67	1	.182	2	.031	9	0	3	0	9	0	15
142		min	-1140.948	3	.041	15	-.289	3	0	2	0	3	0	4
143	15	max	793.767	1	.153	2	.031	9	0	3	0	9	0	15
144		min	-1140.875	3	.032	15	-.289	3	0	2	0	3	0	4
145	16	max	793.863	1	.123	2	.031	9	0	3	0	9	0	15
146		min	-1140.803	3	.023	15	-.289	3	0	2	0	3	0	4
147	17	max	793.959	1	.094	2	.031	9	0	3	0	9	0	15
148		min	-1140.731	3	.002	9	-.289	3	0	2	0	3	0	4
149	18	max	794.056	1	.064	2	.031	9	0	3	0	9	0	15
150		min	-1140.659	3	-.023	9	-.289	3	0	2	0	3	0	4
151	19	max	794.152	1	.035	2	.031	9	0	3	0	9	0	15





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1140.586	3	-.047	9	-.289	3	0	2	0	3	0	4
153		max	187.722	2	1.813	4	0	2	0	9	0	1	0	4
154		min	-91.09	9	.427	15	-.018	1	0	3	0	3	0	15
155		2 max	187.655	2	1.635	4	0	2	0	9	0	1	0	2
156		min	-91.146	9	.385	15	-.018	1	0	3	0	3	0	15
157		3 max	187.588	2	1.457	4	0	2	0	9	0	1	0	2
158		min	-91.201	9	.343	15	-.018	1	0	3	0	3	0	9
159		4 max	187.521	2	1.279	4	0	2	0	9	0	1	0	10
160		min	-91.257	9	.302	15	-.018	1	0	3	0	3	0	14
161		5 max	187.454	2	1.101	4	0	2	0	9	0	1	0	15
162		min	-91.313	9	.26	15	-.018	1	0	3	0	3	0	4
163		6 max	187.387	2	.923	4	0	2	0	9	0	1	0	15
164		min	-91.369	9	.218	15	-.018	1	0	3	0	3	0	4
165		7 max	187.32	2	.745	4	0	2	0	9	0	1	0	15
166		min	-91.425	9	.176	15	-.018	1	0	3	0	3	0	4
167		8 max	187.252	2	.567	4	0	2	0	9	0	1	0	15
168		min	-91.481	9	.134	15	-.018	1	0	3	0	3	0	4
169		9 max	187.185	2	.389	4	0	2	0	9	0	1	0	15
170		min	-91.537	9	.092	15	-.018	1	0	3	0	3	-.001	4
171	10	max	187.118	2	.21	4	0	2	0	9	0	1	0	15
172		min	-91.593	9	.051	15	-.018	1	0	3	0	3	-.001	4
173	11	max	187.051	2	.057	2	0	2	0	9	0	1	0	15
174		min	-91.649	9	.001	9	-.018	1	0	3	0	3	-.001	4
175	12	max	186.984	2	-.033	15	0	2	0	9	0	1	0	15
176		min	-91.705	9	-.146	4	-.018	1	0	3	0	3	-.001	4
177	13	max	186.917	2	-.075	15	0	2	0	9	0	1	0	15
178		min	-91.761	9	-.324	4	-.018	1	0	3	0	3	-.001	4
179	14	max	186.85	2	-.117	15	0	2	0	9	0	1	0	15
180		min	-91.816	9	-.502	4	-.018	1	0	3	0	3	-.001	4
181	15	max	186.783	2	-.159	15	0	2	0	9	0	1	0	15
182		min	-91.872	9	-.68	4	-.018	1	0	3	0	3	0	4
183	16	max	186.716	2	-.201	15	0	2	0	9	0	1	0	15
184		min	-91.928	9	-.858	4	-.018	1	0	3	0	3	0	4
185	17	max	186.649	2	-.242	15	0	2	0	9	0	9	0	15
186		min	-91.984	9	-1.036	4	-.018	1	0	3	0	3	0	4
187	18	max	186.582	2	-.284	15	0	2	0	9	0	9	0	15
188		min	-92.04	9	-1.214	4	-.018	1	0	3	0	3	0	4
189	19	max	186.514	2	-.326	15	0	2	0	9	0	9	0	1
190		min	-92.096	9	-1.392	4	-.018	1	0	3	0	3	0	1
191	M8	max	1060.542	2	0	1	.091	9	0	1	0	2	0	1
192		min	-403.345	3	0	1	-.426	3	0	1	0	3	0	1
193	2	max	1060.607	2	0	1	.091	9	0	1	0	9	0	1
194		min	-403.296	3	0	1	-.426	3	0	1	0	3	0	1
195	3	max	1060.671	2	0	1	.091	9	0	1	0	9	0	1
196		min	-403.248	3	0	1	-.426	3	0	1	0	3	0	1
197	4	max	1060.736	2	0	1	.091	9	0	1	0	9	0	1
198		min	-403.199	3	0	1	-.426	3	0	1	0	3	0	1
199	5	max	1060.801	2	0	1	.091	9	0	1	0	9	0	1
200		min	-403.151	3	0	1	-.426	3	0	1	0	3	0	1
201	6	max	1060.865	2	0	1	.091	9	0	1	0	9	0	1
202		min	-403.102	3	0	1	-.426	3	0	1	0	3	0	1
203	7	max	1060.93	2	0	1	.091	9	0	1	0	9	0	1
204		min	-403.054	3	0	1	-.426	3	0	1	0	3	0	1
205	8	max	1060.995	2	0	1	.091	9	0	1	0	9	0	1
206		min	-403.005	3	0	1	-.426	3	0	1	0	3	0	1
207	9	max	1061.059	2	0	1	.091	9	0	1	0	9	0	1
208		min	-402.957	3	0	1	-.426	3	0	1	0	3	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	1061.124	2	0	1	.091	9	0	1	0	9	0	1
210		min	-402.908	3	0	1	-.426	3	0	1	0	3	0	1
211	11	max	1061.189	2	0	1	.091	9	0	1	0	9	0	1
212		min	-402.86	3	0	1	-.426	3	0	1	0	3	0	1
213	12	max	1061.254	2	0	1	.091	9	0	1	0	9	0	1
214		min	-402.811	3	0	1	-.426	3	0	1	0	3	0	1
215	13	max	1061.318	2	0	1	.091	9	0	1	0	9	0	1
216		min	-402.763	3	0	1	-.426	3	0	1	0	3	0	1
217	14	max	1061.383	2	0	1	.091	9	0	1	0	9	0	1
218		min	-402.714	3	0	1	-.426	3	0	1	0	3	0	1
219	15	max	1061.448	2	0	1	.091	9	0	1	0	9	0	1
220		min	-402.666	3	0	1	-.426	3	0	1	0	3	0	1
221	16	max	1061.512	2	0	1	.091	9	0	1	0	9	0	1
222		min	-402.617	3	0	1	-.426	3	0	1	0	3	0	1
223	17	max	1061.577	2	0	1	.091	9	0	1	0	9	0	1
224		min	-402.568	3	0	1	-.426	3	0	1	0	3	0	1
225	18	max	1061.642	2	0	1	.091	9	0	1	0	9	0	1
226		min	-402.52	3	0	1	-.426	3	0	1	0	3	0	1
227	19	max	1061.707	2	0	1	.091	9	0	1	0	9	0	1
228		min	-402.471	3	0	1	-.426	3	0	1	0	3	0	1
229	M10	1	max 252.241	1	.669	4	0	10	0	1	0	1	0	1
230		min	-327.436	3	.158	15	-.105	1	0	3	0	3	0	1
231	2	max	252.338	1	.631	4	0	10	0	1	0	1	0	15
232		min	-327.363	3	.149	15	-.105	1	0	3	0	3	0	4
233	3	max	252.434	1	.593	4	0	10	0	1	0	1	0	15
234		min	-327.291	3	.14	15	-.105	1	0	3	0	3	0	4
235	4	max	252.53	1	.555	4	0	10	0	1	0	10	0	15
236		min	-327.219	3	.131	15	-.105	1	0	3	0	3	0	4
237	5	max	252.627	1	.517	4	0	10	0	1	0	10	0	15
238		min	-327.146	3	.122	15	-.105	1	0	3	0	3	0	4
239	6	max	252.723	1	.479	4	0	10	0	1	0	10	0	15
240		min	-327.074	3	.113	15	-.105	1	0	3	0	3	0	4
241	7	max	252.82	1	.442	4	0	10	0	1	0	10	0	15
242		min	-327.002	3	.105	15	-.105	1	0	3	0	3	0	4
243	8	max	252.916	1	.404	4	0	10	0	1	0	10	0	15
244		min	-326.93	3	.096	15	-.105	1	0	3	0	3	0	4
245	9	max	253.012	1	.366	4	0	10	0	1	0	10	0	15
246		min	-326.857	3	.087	15	-.105	1	0	3	0	3	0	4
247	10	max	253.109	1	.328	4	0	10	0	1	0	10	0	15
248		min	-326.785	3	.078	15	-.105	1	0	3	0	3	0	4
249	11	max	253.205	1	.29	4	0	10	0	1	0	10	0	15
250		min	-326.713	3	.069	15	-.105	1	0	3	0	3	0	4
251	12	max	253.301	1	.252	4	0	10	0	1	0	10	0	15
252		min	-326.641	3	.06	15	-.105	1	0	3	0	3	0	4
253	13	max	253.398	1	.215	4	0	10	0	1	0	10	0	15
254		min	-326.568	3	.051	15	-.105	1	0	3	0	3	0	4
255	14	max	253.494	1	.177	4	0	10	0	1	0	10	0	15
256		min	-326.496	3	.042	15	-.105	1	0	3	0	3	0	4
257	15	max	253.59	1	.139	4	0	10	0	1	0	10	0	15
258		min	-326.424	3	.033	15	-.105	1	0	3	0	3	0	4
259	16	max	253.687	1	.101	4	0	10	0	1	0	10	0	15
260		min	-326.351	3	.025	15	-.105	1	0	3	0	3	0	4
261	17	max	253.783	1	.073	3	0	10	0	1	0	10	0	15
262		min	-326.279	3	.016	15	-.105	1	0	3	0	3	0	4
263	18	max	253.88	1	.051	3	0	10	0	1	0	10	0	15
264		min	-326.207	3	.002	9	-.105	1	0	3	0	3	0	4
265	19	max	253.976	1	.028	3	0	10	0	1	0	10	0	15



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-326.135	3	-.023	9	-.105	1	0	3	0	3	0	4
267	M11	1	max	55.644	2	1.817	4	.144	1	0	3	0	3	0	4
268			min	-48.457	9	.428	15	-.024	3	0	10	0	1	0	15
269		2	max	55.577	2	1.639	4	.144	1	0	3	0	3	0	4
270			min	-48.513	9	.386	15	-.024	3	0	10	0	1	0	15
271		3	max	55.509	2	1.461	4	.144	1	0	3	0	3	0	2
272			min	-48.569	9	.344	15	-.024	3	0	10	0	1	0	3
273		4	max	55.442	2	1.283	4	.144	1	0	3	0	3	0	15
274			min	-48.625	9	.302	15	-.024	3	0	10	0	1	0	4
275		5	max	55.375	2	1.105	4	.144	1	0	3	0	3	0	15
276			min	-48.681	9	.26	15	-.024	3	0	10	0	1	0	4
277		6	max	55.308	2	.927	4	.144	1	0	3	0	3	0	15
278			min	-48.737	9	.218	15	-.024	3	0	10	0	1	0	4
279		7	max	55.241	2	.749	4	.144	1	0	3	0	3	0	15
280			min	-48.793	9	.177	15	-.024	3	0	10	0	1	0	4
281		8	max	55.174	2	.571	4	.144	1	0	3	0	3	0	15
282			min	-48.849	9	.135	15	-.024	3	0	10	0	1	0	4
283		9	max	55.107	2	.393	4	.144	1	0	3	0	3	0	15
284			min	-48.905	9	.093	15	-.024	3	0	10	0	1	-.001	4
285		10	max	55.04	2	.215	4	.144	1	0	3	0	3	0	15
286			min	-48.961	9	.051	15	-.024	3	0	10	0	1	-.001	4
287		11	max	54.973	2	.04	2	.144	1	0	3	0	3	0	15
288			min	-49.016	9	.003	3	-.024	3	0	10	0	1	-.001	4
289		12	max	54.906	2	-.033	15	.144	1	0	3	0	3	0	15
290			min	-49.072	9	-.141	4	-.024	3	0	10	0	1	-.001	4
291		13	max	54.838	2	-.075	15	.144	1	0	3	0	3	0	15
292			min	-49.128	9	-.319	4	-.024	3	0	10	0	1	-.001	4
293		14	max	54.771	2	-.116	15	.144	1	0	3	0	3	0	15
294			min	-49.184	9	-.497	4	-.024	3	0	10	0	10	-.001	4
295		15	max	54.704	2	-.158	15	.144	1	0	3	0	3	0	15
296			min	-49.24	9	-.675	4	-.024	3	0	10	0	10	0	4
297		16	max	54.637	2	-.2	15	.144	1	0	3	0	3	0	15
298			min	-49.296	9	-.854	4	-.024	3	0	10	0	10	0	4
299		17	max	54.57	2	-.242	15	.144	1	0	3	0	3	0	15
300			min	-49.352	9	-1.032	4	-.024	3	0	10	0	10	0	4
301		18	max	54.503	2	-.284	15	.144	1	0	3	0	3	0	15
302			min	-49.408	9	-1.21	4	-.024	3	0	10	0	10	0	4
303		19	max	54.436	2	-.326	15	.144	1	0	3	0	3	0	1
304			min	-49.464	9	-1.388	4	-.024	3	0	10	0	10	0	1
305	M12	1	max	347.927	1	0	1	.507	1	0	1	0	2	0	1
306			min	-118.873	3	0	1	-.022	10	0	1	0	3	0	1
307		2	max	347.992	1	0	1	.507	1	0	1	0	1	0	1
308			min	-118.825	3	0	1	-.022	10	0	1	0	15	0	1
309		3	max	348.056	1	0	1	.507	1	0	1	0	1	0	1
310			min	-118.776	3	0	1	-.022	10	0	1	0	10	0	1
311		4	max	348.121	1	0	1	.507	1	0	1	0	1	0	1
312			min	-118.728	3	0	1	-.022	10	0	1	0	10	0	1
313		5	max	348.186	1	0	1	.507	1	0	1	0	1	0	1
314			min	-118.679	3	0	1	-.022	10	0	1	0	10	0	1
315		6	max	348.25	1	0	1	.507	1	0	1	0	1	0	1
316			min	-118.631	3	0	1	-.022	10	0	1	0	10	0	1
317		7	max	348.315	1	0	1	.507	1	0	1	0	1	0	1
318			min	-118.582	3	0	1	-.022	10	0	1	0	10	0	1
319		8	max	348.38	1	0	1	.507	1	0	1	0	1	0	1
320			min	-118.534	3	0	1	-.022	10	0	1	0	10	0	1
321		9	max	348.445	1	0	1	.507	1	0	1	0	1	0	1
322			min	-118.485	3	0	1	-.022	10	0	1	0	10	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-51.944	1	-166.556	3	-12.673	1	0	2	-.024	1	0	3
381	M5	max	129.039	1	1103.768	3	0	11	0	9	.008	3	0	3
382		min	-1.994	3	-811.527	1	-53.112	3	0	3	0	11	0	2
383		max	129.111	1	1103.566	3	0	11	0	9	0	9	.175	1
384		min	-1.94	3	-811.797	1	-53.112	3	0	3	-.004	3	-.239	3
385		max	157.424	1	6.266	9	5.717	3	0	3	0	9	.348	1
386		min	-42.944	3	-75.859	3	-.101	9	0	9	-.015	3	-.473	3
387		max	157.496	1	6.041	9	5.717	3	0	3	0	9	.353	1
388		min	-42.89	3	-76.061	3	-.101	9	0	9	-.013	3	-.456	3
389		max	157.568	1	5.816	9	5.717	3	0	3	0	9	.357	1
390		min	-42.836	3	-76.263	3	-.101	9	0	9	-.012	3	-.44	3
391		max	157.64	1	5.591	9	5.717	3	0	3	0	9	.362	1
392		min	-42.782	3	-76.466	3	-.101	9	0	9	-.011	3	-.423	3
393		max	157.713	1	5.367	9	5.717	3	0	3	0	9	.372	2
394		min	-42.727	3	-76.668	3	-.101	9	0	9	-.01	3	-.407	3
395		max	157.785	1	5.142	9	5.717	3	0	3	0	9	.384	2
396		min	-42.673	3	-76.87	3	-.101	9	0	9	-.008	3	-.39	3
397		max	157.857	1	4.917	9	5.717	3	0	3	0	1	.396	2
398		min	-42.619	3	-77.072	3	-.101	9	0	9	-.007	3	-.373	3
399		max	157.929	1	4.692	9	5.717	3	0	3	0	1	.408	2
400		min	-42.565	3	-77.275	3	-.101	9	0	9	-.006	3	-.357	3
401		max	158.002	1	4.468	9	5.717	3	0	3	0	2	.42	2
402		min	-42.511	3	-77.477	3	-.101	9	0	9	-.005	3	-.34	3
403		max	158.074	1	4.243	9	5.717	3	0	3	0	2	.432	2
404		min	-42.456	3	-77.679	3	-.101	9	0	9	-.004	3	-.323	3
405		max	158.146	1	4.018	9	5.717	3	0	3	0	11	.445	2
406		min	-42.402	3	-77.882	3	-.101	9	0	9	-.002	3	-.306	3
407		max	158.219	1	3.793	9	5.717	3	0	3	0	11	.457	2
408		min	-42.348	3	-78.084	3	-.101	9	0	9	-.001	3	-.289	3
409		max	158.291	1	3.568	9	5.717	3	0	3	0	3	.469	2
410		min	-42.294	3	-78.286	3	-.101	9	0	9	0	9	-.272	3
411		max	221.612	2	65.832	2	5.693	3	0	3	0	3	.481	2
412		min	-106.368	3	-134.086	3	-.103	9	0	2	0	9	-.255	3
413		max	221.684	2	65.562	2	5.693	3	0	3	.002	3	.467	2
414		min	-106.314	3	-134.289	3	-.103	9	0	2	0	9	-.226	3
415		max	-1.673	12	1086.514	2	5.261	3	0	3	.003	3	.235	2
416		min	-129.225	1	-524.935	3	-.02	1	0	9	0	9	-.113	3
417		max	-1.636	12	1086.244	2	5.261	3	0	3	.004	3	0	3
418		min	-129.153	1	-525.138	3	-.02	1	0	9	0	9	0	2
419	M9	max	51.961	1	343.539	3	55.736	3	0	3	0	10	0	2
420		min	1.619	15	-254.01	1	-.383	10	0	1	-.024	1	0	3
421		max	52.034	1	343.337	3	55.736	3	0	3	0	10	.055	1
422		min	1.641	15	-254.28	1	-.383	10	0	1	-.021	1	-.075	3
423		max	61.704	1	4.049	9	12.061	1	0	1	.012	3	.109	1
424		min	-6.145	3	-21.52	3	-2.562	3	0	5	-.018	1	-.148	3
425		max	61.776	1	3.824	9	12.061	1	0	1	.011	3	.11	1
426		min	-6.091	3	-21.722	3	-2.562	3	0	5	-.016	1	-.143	3
427		max	61.848	1	3.599	9	12.061	1	0	1	.01	3	.111	1
428		min	-6.036	3	-21.924	3	-2.562	3	0	5	-.013	1	-.138	3
429		max	61.92	1	3.375	9	12.061	1	0	1	.01	3	.113	2
430		min	-5.982	3	-22.126	3	-2.562	3	0	5	-.011	1	-.133	3
431		max	61.993	1	3.15	9	12.061	1	0	1	.009	3	.116	2
432		min	-5.928	3	-22.329	3	-2.562	3	0	5	-.008	1	-.129	3
433		max	62.065	1	2.925	9	12.061	1	0	1	.009	3	.12	2
434		min	-5.874	3	-22.531	3	-2.562	3	0	5	-.005	1	-.124	3
435		max	62.137	1	2.7	9	12.061	1	0	1	.008	3	.123	2
436		min	-5.82	3	-22.733	3	-2.562	3	0	5	-.003	1	-.119	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	62.209	1	2.476	9	12.061	1	0	1	.008	3	.127	2
438		min	-5.765	3	-22.936	3	-2.562	3	0	5	0	1	-.114	3
439	11	max	62.282	1	2.251	9	12.061	1	0	1	.007	3	.131	2
440		min	-5.711	3	-23.138	3	-2.562	3	0	5	0	10	-.109	3
441	12	max	62.354	1	2.026	9	12.061	1	0	1	.007	3	.135	2
442		min	-5.657	3	-23.34	3	-2.562	3	0	5	0	10	-.104	3
443	13	max	62.426	1	1.801	9	12.061	1	0	1	.008	1	.138	2
444		min	-5.603	3	-23.543	3	-2.562	3	0	5	0	10	-.099	3
445	14	max	62.498	1	1.577	9	12.061	1	0	1	.01	1	.142	2
446		min	-5.549	3	-23.745	3	-2.562	3	0	5	0	10	-.094	3
447	15	max	62.571	1	1.352	9	12.061	1	0	1	.013	1	.146	2
448		min	-5.494	3	-23.947	3	-2.562	3	0	5	0	10	-.088	3
449	16	max	69.291	2	16.221	2	12.197	1	0	10	.016	1	.15	2
450		min	-35.419	3	-51.041	3	-2.577	3	0	3	0	10	-.083	3
451	17	max	69.363	2	15.951	2	12.197	1	0	10	.018	1	.147	2
452		min	-35.365	3	-51.244	3	-2.577	3	0	3	0	10	-.072	3
453	18	max	-1.639	15	340.097	2	12.694	1	0	2	.021	1	.074	2
454		min	-51.971	1	-166.348	3	-2.242	3	0	3	0	10	-.036	3
455	19	max	-1.618	15	339.827	2	12.694	1	0	2	.024	1	0	2
456		min	-51.899	1	-166.55	3	-2.242	3	0	3	0	10	0	3
457	M13	1	max	55.734	3	253.858	1	-1.619	15	0	.024	1	0	1
458		min	-.383	10	-343.571	3	-51.959	1	0	3	0	10	0	3
459	2	max	55.734	3	180.64	1	-1.226	15	0	2	.01	3	.131	3
460		min	-.383	10	-244.078	3	-39.015	1	0	3	-.002	10	-.097	1
461	3	max	55.734	3	107.423	1	-.682	10	0	2	.007	3	.217	3
462		min	-.383	10	-144.585	3	-26.071	1	0	3	-.011	1	-.161	1
463	4	max	55.734	3	34.205	1	.404	10	0	2	.005	3	.259	3
464		min	-.383	10	-45.092	3	-13.127	1	0	3	-.019	1	-.192	1
465	5	max	55.734	3	54.402	3	2.116	2	0	2	.004	3	.257	3
466		min	-.383	10	-39.013	1	-3.548	3	0	3	-.022	1	-.191	1
467	6	max	55.734	3	153.895	3	12.762	1	0	2	.002	3	.211	3
468		min	-.383	10	-112.23	1	-2.975	3	0	3	-.02	1	-.157	1
469	7	max	55.734	3	253.388	3	25.706	1	0	2	0	3	.12	3
470		min	-.383	10	-185.448	1	-2.403	3	0	3	-.011	1	-.091	1
471	8	max	55.734	3	352.881	3	38.65	1	0	2	.004	2	.007	1
472		min	-.383	10	-258.665	1	-1.831	3	0	3	0	12	-.014	3
473	9	max	55.734	3	452.374	3	51.595	1	0	2	.023	1	.139	1
474		min	-.383	10	-331.883	1	-1.259	3	0	3	0	3	-.193	3
475	10	max	55.734	3	-8.127	15	64.539	1	0	2	.049	1	.302	1
476		min	-.383	10	-551.867	3	.606	12	0	3	-.009	3	-.417	3
477	11	max	12.266	1	331.883	1	1.891	3	0	3	.023	1	.139	1
478		min	-.383	10	-452.374	3	-51.561	1	0	2	-.008	3	-.193	3
479	12	max	12.266	1	258.665	1	2.464	3	0	3	.004	2	.007	1
480		min	-.383	10	-352.881	3	-38.617	1	0	2	-.007	3	-.014	3
481	13	max	12.266	1	185.448	1	3.036	3	0	3	0	10	.12	3
482		min	-.383	10	-253.388	3	-25.673	1	0	2	-.011	1	-.091	1
483	14	max	12.266	1	112.23	1	3.608	3	0	3	0	15	.211	3
484		min	-.383	10	-153.895	3	-12.729	1	0	2	-.02	1	-.157	1
485	15	max	12.266	1	39.013	1	4.18	3	0	3	0	15	.257	3
486		min	-.383	10	-54.401	3	-2.116	2	0	2	-.022	1	-.191	1
487	16	max	12.266	1	45.092	3	13.16	1	0	3	0	12	.259	3
488		min	-.383	10	-34.205	1	-.404	10	0	2	-.019	1	-.192	1
489	17	max	12.266	1	144.585	3	26.104	1	0	3	.002	3	.217	3
490		min	-.383	10	-107.423	1	.682	10	0	2	-.011	1	-.161	1
491	18	max	12.266	1	244.078	3	39.048	1	0	3	.004	3	.131	3
492		min	-.383	10	-180.64	1	1.229	15	0	2	-.002	10	-.097	1
493	19	max	12.266	1	343.571	3	51.992	1	0	3	.024	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-383	10	-253.858	1	1.622	15	0	2	0	10	0	3
495	M16	1	max	2.243	3	339.893	2	-1.618	15	0	3	.024	1	0	2
496			min	-12.678	1	-166.564	3	-51.901	1	0	2	0	10	0	3
497		2	max	2.243	3	241.743	2	-1.224	15	0	3	.004	1	.063	3
498			min	-12.678	1	-118.882	3	-38.957	1	0	2	-.002	10	-.129	2
499		3	max	2.243	3	143.593	2	-.661	10	0	3	0	12	.106	3
500			min	-12.678	1	-71.199	3	-26.013	1	0	2	-.011	1	-.215	2
501		4	max	2.243	3	45.443	2	.425	10	0	3	0	15	.127	3
502			min	-12.678	1	-23.517	3	-13.069	1	0	2	-.019	1	-.257	2
503		5	max	2.243	3	24.166	3	2.157	2	0	3	0	15	.127	3
504			min	-12.678	1	-52.707	2	-2.231	3	0	2	-.022	1	-.255	2
505		6	max	2.243	3	71.848	3	12.82	1	0	3	0	15	.105	3
506			min	-12.678	1	-150.857	2	-1.659	3	0	2	-.019	1	-.21	2
507		7	max	2.243	3	119.531	3	25.764	1	0	3	0	10	.063	3
508			min	-12.678	1	-249.007	2	-1.086	3	0	2	-.011	1	-.121	2
509		8	max	2.243	3	167.213	3	38.708	1	0	3	.004	2	.011	2
510			min	-12.678	1	-347.157	2	-.514	3	0	2	-.005	3	-.001	3
511		9	max	2.243	3	214.896	3	51.652	1	0	3	.024	1	.187	2
512			min	-12.678	1	-445.307	2	.058	3	0	2	-.005	3	-.086	3
513		10	max	.409	10	-8.125	15	64.597	1	0	15	.049	1	.407	2
514			min	-12.678	1	-543.457	2	-1.481	3	0	2	-.004	3	-.192	3
515		11	max	.409	10	445.307	2	-.691	12	0	2	.023	1	.187	2
516			min	-12.658	1	-214.896	3	-51.607	1	0	3	0	3	-.086	3
517		12	max	.409	10	347.157	2	-.309	12	0	2	.004	2	.011	2
518			min	-12.658	1	-167.213	3	-38.663	1	0	3	0	3	-.001	3
519		13	max	.409	10	249.007	2	.236	3	0	2	0	10	.063	3
520			min	-12.658	1	-119.531	3	-25.719	1	0	3	-.011	1	-.121	2
521		14	max	.409	10	150.857	2	.808	3	0	2	0	12	.105	3
522			min	-12.658	1	-71.848	3	-12.774	1	0	3	-.019	1	-.21	2
523		15	max	.409	10	52.707	2	1.38	3	0	2	0	3	.127	3
524			min	-12.658	1	-24.166	3	-2.157	2	0	3	-.022	1	-.255	2
525		16	max	.409	10	23.517	3	13.114	1	0	2	0	3	.127	3
526			min	-12.658	1	-45.443	2	-.425	10	0	3	-.019	1	-.257	2
527		17	max	.409	10	71.199	3	26.058	1	0	2	.002	3	.106	3
528			min	-12.658	1	-143.593	2	.661	10	0	3	-.011	1	-.215	2
529		18	max	.409	10	118.882	3	39.002	1	0	2	.004	1	.063	3
530			min	-12.658	1	-241.743	2	1.228	15	0	3	-.002	10	-.129	2
531		19	max	.409	10	166.564	3	51.947	1	0	2	.024	1	0	2
532			min	-12.658	1	-339.893	2	1.621	15	0	3	0	10	0	3
533	M15	1	max	0	1	.824	3	.136	3	0	1	0	1	0	1
534			min	-69.458	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.732	3	.136	3	0	1	0	1	0	1
536			min	-69.512	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.641	3	.136	3	0	1	0	1	0	1
538			min	-69.566	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.549	3	.136	3	0	1	0	1	0	1
540			min	-69.62	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.458	3	.136	3	0	1	0	1	0	1
542			min	-69.674	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.366	3	.136	3	0	1	0	1	0	1
544			min	-69.728	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.275	3	.136	3	0	1	0	3	0	1
546			min	-69.782	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.183	3	.136	3	0	1	0	3	0	1
548			min	-69.836	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.092	3	.136	3	0	1	0	3	0	1
550			min	-69.89	3	0	1	0	1	0	3	0	1	0	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.136	3	0	1	0	3	0	1
552		min	-69.944	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.136	3	0	1	0	3	0	1
554		min	-69.998	3	-.092	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.136	3	0	1	0	3	0	1
556		min	-70.052	3	-.183	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.136	3	0	1	0	3	0	1
558		min	-70.106	3	-.275	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.136	3	0	1	0	3	0	1
560		min	-70.16	3	-.366	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.136	3	0	1	0	3	0	1
562		min	-70.214	3	-.458	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.136	3	0	1	0	3	0	1
564		min	-70.268	3	-.549	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.136	3	0	1	0	3	0	1
566		min	-70.322	3	-.641	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.136	3	0	1	0	3	0	1
568		min	-70.376	3	-.732	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.136	3	0	1	0	3	0	1
570		min	-70.43	3	-.824	3	0	1	0	3	0	1	0	1
571	M16A 1	max	0	2	1.409	4	.046	1	0	3	0	3	0	1
572		min	-69.18	3	0	2	-.053	3	0	1	0	1	0	1
573	2	max	0	2	1.253	4	.046	1	0	3	0	3	0	2
574		min	-69.126	3	0	2	-.053	3	0	1	0	1	0	4
575	3	max	0	2	1.096	4	.046	1	0	3	0	3	0	2
576		min	-69.072	3	0	2	-.053	3	0	1	0	1	0	4
577	4	max	0	2	.939	4	.046	1	0	3	0	3	0	2
578		min	-69.018	3	0	2	-.053	3	0	1	0	1	0	4
579	5	max	0	2	.783	4	.046	1	0	3	0	3	0	2
580		min	-68.964	3	0	2	-.053	3	0	1	0	1	-.001	4
581	6	max	0	2	.626	4	.046	1	0	3	0	3	0	2
582		min	-68.91	3	0	2	-.053	3	0	1	0	1	-.001	4
583	7	max	0	2	.47	4	.046	1	0	3	0	3	0	2
584		min	-68.856	3	0	2	-.053	3	0	1	0	1	-.001	4
585	8	max	0	2	.313	4	.046	1	0	3	0	3	0	2
586		min	-68.802	3	0	2	-.053	3	0	1	0	1	-.002	4
587	9	max	0	2	.157	4	.046	1	0	3	0	3	0	2
588		min	-68.748	3	0	2	-.053	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.046	1	0	3	0	3	0	2
590		min	-68.694	3	0	1	-.053	3	0	1	0	1	-.002	4
591	11	max	.018	13	0	2	.046	1	0	3	0	3	0	2
592		min	-68.64	3	-.157	4	-.053	3	0	1	0	1	-.002	4
593	12	max	.092	13	0	2	.046	1	0	3	0	3	0	2
594		min	-68.586	3	-.313	4	-.053	3	0	1	0	1	-.002	4
595	13	max	.167	13	0	2	.046	1	0	3	0	1	0	2
596		min	-68.532	3	-.47	4	-.053	3	0	1	0	4	-.001	4
597	14	max	.241	13	0	2	.046	1	0	3	0	1	0	2
598		min	-68.478	3	-.626	4	-.053	3	0	1	0	3	-.001	4
599	15	max	.315	13	0	2	.046	1	0	3	0	1	0	2
600		min	-68.424	3	-.783	4	-.053	3	0	1	0	3	-.001	4
601	16	max	.407	4	0	2	.046	1	0	3	0	1	0	2
602		min	-68.37	3	-.939	4	-.053	3	0	1	0	3	0	4
603	17	max	.499	4	0	2	.046	1	0	3	0	1	0	2
604		min	-68.316	3	-1.096	4	-.053	3	0	1	0	3	0	4
605	18	max	.592	4	0	2	.046	1	0	3	0	1	0	2
606		min	-68.262	3	-1.253	4	-.053	3	0	1	0	3	0	4
607	19	max	.684	4	0	2	.046	1	0	3	0	1	0	1





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-68.208	3	-1.409	4	-.053	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.005	2	.002	1	5.023e-6	10	NC	3	NC	1	
2		min	-0.003	3	-.005	3	-.001	3	-1.795e-4	1	5552.937	2	NC	1		
3		2	max	.002	1	.005	2	.002	1	4.782e-6	10	NC	3	NC	1	
4		min	-0.003	3	-.004	3	-.001	3	-1.718e-4	1	6034.955	2	NC	1		
5		3	max	.002	1	.005	2	.002	1	4.541e-6	10	NC	1	NC	1	
6		min	-.002	3	-.004	3	-.001	3	-1.642e-4	1	6604.045	2	NC	1		
7		4	max	.002	1	.004	2	.001	1	4.301e-6	10	NC	1	NC	1	
8		min	-.002	3	-.004	3	-.001	3	-1.565e-4	1	7280.82	2	NC	1		
9		5	max	.001	1	.004	2	.001	1	4.06e-6	10	NC	1	NC	1	
10		min	-.002	3	-.004	3	0	3	-1.488e-4	1	8092.643	2	NC	1		
11		6	max	.001	1	.003	2	.001	1	3.819e-6	10	NC	1	NC	1	
12		min	-.002	3	-.004	3	0	3	-1.411e-4	1	9076.45	2	NC	1		
13		7	max	.001	1	.003	2	0	1	3.578e-6	10	NC	1	NC	1	
14		min	-.002	3	-.003	3	0	3	-1.334e-4	1	NC	1	NC	1		
15		8	max	.001	1	.003	2	0	1	3.337e-6	10	NC	1	NC	1	
16		min	-.002	3	-.003	3	0	3	-1.257e-4	1	NC	1	NC	1		
17		9	max	.001	1	.002	2	0	1	3.097e-6	10	NC	1	NC	1	
18		min	-.002	3	-.003	3	0	3	-1.18e-4	1	NC	1	NC	1		
19		10	max	0	1	.002	2	0	1	2.856e-6	10	NC	1	NC	1	
20		min	-.001	3	-.003	3	0	3	-1.104e-4	1	NC	1	NC	1		
21	M3	11	max	0	1	.002	2	0	1	2.615e-6	10	NC	1	NC	1	
22		min	-.001	3	-.003	3	0	3	-1.027e-4	1	NC	1	NC	1		
23		12	max	0	1	.001	2	0	1	2.374e-6	10	NC	1	NC	1	
24		min	-.001	3	-.002	3	0	3	-9.499e-5	1	NC	1	NC	1		
25		13	max	0	1	.001	2	0	1	2.133e-6	10	NC	1	NC	1	
26		min	0	3	-.002	3	0	3	-8.73e-5	1	NC	1	NC	1		
27		14	max	0	1	0	2	0	1	1.893e-6	10	NC	1	NC	1	
28		min	0	3	-.002	3	0	3	-7.962e-5	1	NC	1	NC	1		
29		15	max	0	1	0	2	0	1	1.652e-6	10	NC	1	NC	1	
30		min	0	3	-.001	3	0	3	-7.193e-5	1	NC	1	NC	1		
31		16	max	0	1	0	2	0	1	1.411e-6	10	NC	1	NC	1	
32		min	0	3	-.001	3	0	3	-6.425e-5	1	NC	1	NC	1		
33		17	max	0	1	0	2	0	1	1.17e-6	10	NC	1	NC	1	
34		min	0	3	0	3	0	3	-5.656e-5	1	NC	1	NC	1		
35		18	max	0	1	0	2	0	1	9.293e-7	10	NC	1	NC	1	
36		min	0	3	0	3	0	3	-4.888e-5	1	NC	1	NC	1		
37		19	max	0	1	0	1	0	1	6.885e-7	10	NC	1	NC	1	
38		min	0	1	0	1	0	1	-4.119e-5	1	NC	1	NC	1		
39		M3	1	max	0	1	0	1	0	1	1.879e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-3.174e-7	10	NC	1	NC	1	
41	2		max	0	9	0	2	0	10	2.611e-5	1	NC	1	NC	1	
42	min		0	2	0	3	0	1	-6.051e-7	10	NC	1	NC	1		
43	3		max	0	9	0	2	0	10	3.342e-5	1	NC	1	NC	1	
44	min		0	2	-.001	3	0	1	-8.928e-7	10	NC	1	NC	1		
45	4		max	0	9	0	2	0	10	4.073e-5	1	NC	1	NC	1	
46	min		0	2	-.002	3	0	1	-1.18e-6	10	NC	1	NC	1		
47	5		max	0	9	0	2	0	3	4.805e-5	1	NC	1	NC	1	
48	min		0	2	-.003	3	0	9	-1.468e-6	10	NC	1	NC	1		
49	6		max	0	9	0	2	0	3	5.536e-5	1	NC	1	NC	1	
50	min		0	2	-.003	3	0	9	-1.756e-6	10	NC	1	NC	1		
51	7	max	0	9	0	2	0	3	6.268e-5	1	NC	1	NC	1		



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.004	3	0	9	-2.044e-6	10	NC	1	NC	1
53		8	max	0	9	0	2	0	3	6.999e-5	1	NC	1	NC	1
54			min	0	2	-.004	3	0	9	-2.331e-6	10	NC	1	NC	1
55		9	max	0	9	0	2	0	1	7.73e-5	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-2.619e-6	10	NC	1	NC	1
57		10	max	0	9	.001	2	0	1	8.462e-5	1	NC	1	NC	1
58			min	0	2	-.005	3	0	10	-2.907e-6	10	NC	1	NC	1
59		11	max	0	9	.002	2	0	1	9.193e-5	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-3.194e-6	10	NC	1	NC	1
61		12	max	0	9	.002	2	0	1	9.925e-5	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-3.482e-6	10	NC	1	NC	1
63		13	max	0	9	.003	2	0	1	1.066e-4	1	NC	1	NC	1
64			min	0	2	-.006	3	0	10	-3.77e-6	10	NC	1	NC	1
65		14	max	0	9	.003	2	0	1	1.139e-4	1	NC	1	NC	1
66			min	0	2	-.006	3	0	10	-4.057e-6	10	NC	1	NC	1
67		15	max	0	9	.004	2	.001	1	1.212e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-4.345e-6	10	NC	1	NC	1
69		16	max	0	9	.005	2	.001	1	1.285e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-4.633e-6	10	9142.315	2	NC	1
71		17	max	0	9	.006	2	.001	1	1.358e-4	1	NC	1	NC	1
72			min	0	2	-.007	3	0	10	-4.92e-6	10	7770.866	2	NC	1
73		18	max	0	9	.007	2	.001	1	1.431e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	10	-5.208e-6	10	6717.76	2	NC	1
75		19	max	0	9	.008	2	.002	1	1.504e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	10	-5.496e-6	10	5899.813	2	NC	1
77	M4	1	max	.002	1	.006	2	0	10	5.47e-6	10	NC	1	NC	1
78			min	0	3	-.005	3	-.001	1	-1.538e-4	1	NC	1	NC	1
79		2	max	.002	1	.006	2	0	10	5.47e-6	10	NC	1	NC	1
80			min	0	3	-.005	3	-.001	1	-1.538e-4	1	NC	1	NC	1
81		3	max	.001	1	.006	2	0	10	5.47e-6	10	NC	1	NC	1
82			min	0	3	-.004	3	-.001	1	-1.538e-4	1	NC	1	NC	1
83		4	max	.001	1	.005	2	0	10	5.47e-6	10	NC	1	NC	1
84			min	0	3	-.004	3	0	1	-1.538e-4	1	NC	1	NC	1
85		5	max	.001	1	.005	2	0	10	5.47e-6	10	NC	1	NC	1
86			min	0	3	-.004	3	0	1	-1.538e-4	1	NC	1	NC	1
87		6	max	.001	1	.004	2	0	10	5.47e-6	10	NC	1	NC	1
88			min	0	3	-.004	3	0	1	-1.538e-4	1	NC	1	NC	1
89		7	max	.001	1	.004	2	0	10	5.47e-6	10	NC	1	NC	1
90			min	0	3	-.003	3	0	1	-1.538e-4	1	NC	1	NC	1
91		8	max	.001	1	.004	2	0	10	5.47e-6	10	NC	1	NC	1
92			min	0	3	-.003	3	0	1	-1.538e-4	1	NC	1	NC	1
93		9	max	0	1	.003	2	0	10	5.47e-6	10	NC	1	NC	1
94			min	0	3	-.003	3	0	1	-1.538e-4	1	NC	1	NC	1
95		10	max	0	1	.003	2	0	10	5.47e-6	10	NC	1	NC	1
96			min	0	3	-.002	3	0	1	-1.538e-4	1	NC	1	NC	1
97		11	max	0	1	.003	2	0	10	5.47e-6	10	NC	1	NC	1
98			min	0	3	-.002	3	0	1	-1.538e-4	1	NC	1	NC	1
99		12	max	0	1	.002	2	0	10	5.47e-6	10	NC	1	NC	1
100			min	0	3	-.002	3	0	1	-1.538e-4	1	NC	1	NC	1
101		13	max	0	1	.002	2	0	10	5.47e-6	10	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-1.538e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	10	5.47e-6	10	NC	1	NC	1
104			min	0	3	-.001	3	0	1	-1.538e-4	1	NC	1	NC	1
105		15	max	0	1	.001	2	0	10	5.47e-6	10	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-1.538e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	10	5.47e-6	10	NC	1	NC	1
108			min	0	3	0	3	0	1	-1.538e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	10	5.47e-6	10	NC	1	NC	1
110			min	0	3	0	3	0	1	-1.538e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	5.47e-6	10	NC	1	NC	1
112			min	0	3	0	3	0	1	-1.538e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	5.47e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.538e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.018	2	0	9	2.923e-4	3	NC	3	NC	1
116			min	-.008	3	-.013	3	-.004	3	-8.849e-8	2	1710.596	2	7389.299	3
117		2	max	.006	1	.016	2	0	9	2.858e-4	3	NC	3	NC	1
118			min	-.008	3	-.013	3	-.004	3	-8.382e-8	2	1826.677	2	7912.218	3
119		3	max	.005	1	.015	2	0	9	2.794e-4	3	NC	3	NC	1
120			min	-.008	3	-.012	3	-.004	3	-8.487e-7	1	1959.238	2	8525.052	3
121		4	max	.005	1	.014	2	0	9	2.73e-4	3	NC	3	NC	1
122			min	-.007	3	-.011	3	-.003	3	-2.34e-6	1	2111.587	2	9247.926	3
123		5	max	.005	1	.013	2	0	9	2.666e-4	3	NC	3	NC	1
124			min	-.007	3	-.011	3	-.003	3	-3.832e-6	1	2287.984	2	NC	1
125		6	max	.004	1	.012	2	0	9	2.602e-4	3	NC	3	NC	1
126			min	-.006	3	-.01	3	-.003	3	-5.323e-6	1	2494.011	2	NC	1
127		7	max	.004	1	.011	2	0	9	2.537e-4	3	NC	3	NC	1
128			min	-.006	3	-.009	3	-.002	3	-6.815e-6	1	2737.115	2	NC	1
129		8	max	.004	1	.01	2	0	9	2.473e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.002	3	-8.307e-6	1	3027.46	2	NC	1
131		9	max	.003	1	.009	2	0	9	2.409e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	-.002	3	-9.798e-6	1	3379.285	2	NC	1
133		10	max	.003	1	.008	2	0	9	2.345e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	-.002	3	-1.129e-5	1	3813.164	2	NC	1
135		11	max	.003	1	.007	2	0	9	2.281e-4	3	NC	3	NC	1
136			min	-.004	3	-.007	3	-.001	3	-1.278e-5	1	4359.965	2	NC	1
137		12	max	.002	1	.006	2	0	9	2.216e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	-.001	3	-1.427e-5	1	5068.207	2	NC	1
139		13	max	.002	1	.005	2	0	9	2.152e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-1.576e-5	1	6018.757	2	NC	1
141		14	max	.002	1	.004	2	0	9	2.088e-4	3	NC	1	NC	1
142			min	-.002	3	-.004	3	0	3	-1.726e-5	1	7357.189	2	NC	1
143		15	max	.001	1	.003	2	0	9	2.024e-4	3	NC	1	NC	1
144			min	-.002	3	-.003	3	0	3	-1.875e-5	1	9374.65	2	NC	1
145		16	max	0	1	.002	2	0	1	1.96e-4	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-2.024e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.895e-4	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-2.173e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.831e-4	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-2.322e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.767e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-2.471e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.126e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-8.002e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	1.085e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	1	-6.267e-5	3	NC	1	NC	1
157		3	max	0	9	.002	2	0	3	1.044e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-4.532e-5	3	NC	1	NC	1
159		4	max	0	9	.003	2	.001	3	1.004e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-2.797e-5	3	NC	1	NC	1
161		5	max	0	9	.004	2	.001	3	9.631e-6	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-1.062e-5	3	NC	1	NC	1
163		6	max	0	9	.005	2	.002	3	9.225e-6	1	NC	1	NC	1
164			min	0	2	-.007	3	0	1	0	2	8748.546	2	NC	1
165		7	max	0	9	.006	2	.002	3	2.409e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	0	2	7237.41	2	NC	1
167		8	max	0	9	.008	2	.002	3	4.144e-5	3	NC	3	NC	1
168			min	0	2	-.01	3	0	1	0	10	6124.783	2	NC	1
169		9	max	0	9	.009	2	.002	3	5.879e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	0	5	5267.039	2	NC	1
171		10	max	0	9	.01	2	.002	3	7.614e-5	3	NC	3	NC	1
172			min	-.001	2	-.012	3	0	1	-7.249e-8	13	4584.475	2	NC	1
173		11	max	0	9	.011	2	.002	3	9.349e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	0	1	-2.173e-7	4	4029.038	2	NC	1
175		12	max	0	9	.013	2	.003	3	1.108e-4	3	NC	3	NC	1
176			min	-.001	2	-.014	3	0	1	-9.36e-7	9	3569.749	2	NC	1
177		13	max	0	9	.014	2	.003	3	1.282e-4	3	NC	3	NC	1
178			min	-.001	2	-.015	3	0	9	-1.683e-6	9	3185.5	2	NC	1
179		14	max	0	9	.016	2	.003	3	1.455e-4	3	NC	3	NC	1
180			min	-.002	2	-.016	3	0	9	-2.43e-6	9	2861.233	2	NC	1
181		15	max	0	9	.018	2	.002	3	1.629e-4	3	NC	3	NC	1
182			min	-.002	2	-.017	3	0	9	-3.177e-6	9	2585.8	2	NC	1
183		16	max	0	9	.02	2	.002	3	1.803e-4	3	NC	3	NC	1
184			min	-.002	2	-.018	3	0	9	-3.924e-6	9	2350.7	2	NC	1
185		17	max	0	9	.021	2	.002	3	1.976e-4	3	NC	3	NC	1
186			min	-.002	2	-.019	3	0	9	-4.671e-6	9	2149.306	2	NC	1
187		18	max	0	9	.023	2	.002	3	2.15e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	0	9	-5.418e-6	9	1976.363	2	NC	1
189		19	max	.001	9	.025	2	.002	3	2.323e-4	3	NC	3	NC	1
190			min	-.002	2	-.02	3	0	9	-6.165e-6	9	1827.654	2	NC	1
191	M8	1	max	.005	2	.02	2	0	9	-7.541e-8	10	NC	1	NC	1
192			min	-.002	3	-.015	3	-.001	3	-1.845e-4	3	NC	1	NC	1
193		2	max	.005	2	.019	2	0	9	-7.541e-8	10	NC	1	NC	1
194			min	-.002	3	-.014	3	-.001	3	-1.845e-4	3	NC	1	NC	1
195		3	max	.004	2	.018	2	0	9	-7.541e-8	10	NC	1	NC	1
196			min	-.002	3	-.013	3	-.001	3	-1.845e-4	3	NC	1	NC	1
197		4	max	.004	2	.017	2	0	9	-7.541e-8	10	NC	1	NC	1
198			min	-.002	3	-.012	3	-.001	3	-1.845e-4	3	NC	1	NC	1
199		5	max	.004	2	.016	2	0	9	-7.541e-8	10	NC	1	NC	1
200			min	-.001	3	-.012	3	0	3	-1.845e-4	3	NC	1	NC	1
201		6	max	.004	2	.015	2	0	9	-7.541e-8	10	NC	1	NC	1
202			min	-.001	3	-.011	3	0	3	-1.845e-4	3	NC	1	NC	1
203		7	max	.003	2	.013	2	0	9	-7.541e-8	10	NC	1	NC	1
204			min	-.001	3	-.01	3	0	3	-1.845e-4	3	NC	1	NC	1
205		8	max	.003	2	.012	2	0	9	-7.541e-8	10	NC	1	NC	1
206			min	-.001	3	-.009	3	0	3	-1.845e-4	3	NC	1	NC	1
207		9	max	.003	2	.011	2	0	9	-7.541e-8	10	NC	1	NC	1
208			min	-.001	3	-.008	3	0	3	-1.845e-4	3	NC	1	NC	1
209		10	max	.003	2	.01	2	0	9	-7.541e-8	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.845e-4	3	NC	1	NC	1
211		11	max	.002	2	.009	2	0	9	-7.541e-8	10	NC	1	NC	1
212			min	0	3	-.007	3	0	3	-1.845e-4	3	NC	1	NC	1
213		12	max	.002	2	.008	2	0	9	-7.541e-8	10	NC	1	NC	1
214			min	0	3	-.006	3	0	3	-1.845e-4	3	NC	1	NC	1
215		13	max	.002	2	.007	2	0	9	-7.541e-8	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.845e-4	3	NC	1	NC	1
217		14	max	.001	2	.006	2	0	9	-7.541e-8	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.845e-4	3	NC	1	NC	1
219		15	max	.001	2	.004	2	0	9	-7.541e-8	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.845e-4	3	NC	1	NC	1
221		16	max	0	2	.003	2	0	9	-7.541e-8	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.845e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	2	.002	2	0	9	-7.541e-8	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.845e-4	3	NC	1	NC	1
225		18	max	0	2	.001	2	0	9	-7.541e-8	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.845e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-7.541e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.845e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.005	2	0	3	1.835e-4	1	NC	3	NC	1
230			min	-.002	3	-.005	3	-.001	1	-3.959e-4	3	5563.113	2	NC	1
231		2	max	.002	1	.005	2	0	3	1.75e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.001	1	-3.848e-4	3	6046.237	2	NC	1
233		3	max	.002	1	.005	2	0	3	1.664e-4	1	NC	1	NC	1
234			min	-.002	3	-.004	3	-.001	1	-3.736e-4	3	6616.675	2	NC	1
235		4	max	.002	1	.004	2	0	3	1.579e-4	1	NC	1	NC	1
236			min	-.002	3	-.004	3	0	1	-3.624e-4	3	7295.105	2	NC	1
237		5	max	.001	1	.004	2	0	3	1.494e-4	1	NC	1	NC	1
238			min	-.002	3	-.004	3	0	1	-3.513e-4	3	8108.981	2	NC	1
239		6	max	.001	1	.003	2	0	3	1.409e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	0	1	-3.401e-4	3	9095.365	2	NC	1
241		7	max	.001	1	.003	2	0	3	1.324e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	0	1	-3.29e-4	3	NC	1	NC	1
243		8	max	.001	1	.003	2	0	3	1.239e-4	1	NC	1	NC	1
244			min	-.001	3	-.003	3	0	1	-3.178e-4	3	NC	1	NC	1
245		9	max	.001	1	.002	2	0	3	1.154e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-3.067e-4	3	NC	1	NC	1
247		10	max	0	1	.002	2	0	3	1.069e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-2.955e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	9.838e-5	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-2.844e-4	3	NC	1	NC	1
251		12	max	0	1	.001	2	0	3	8.987e-5	1	NC	1	NC	1
252			min	0	3	-.002	3	0	1	-2.732e-4	3	NC	1	NC	1
253		13	max	0	1	0	2	0	3	8.136e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-2.621e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	7.286e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-2.509e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	6.435e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.398e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	5.584e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-2.286e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	4.733e-5	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-2.175e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	3.882e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.063e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.032e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.952e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.909e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.391e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	1	7.188e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.224e-5	1	NC	1	NC	1
271		3	max	0	9	0	2	0	1	5.466e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-3.058e-5	1	NC	1	NC	1
273		4	max	0	9	0	2	0	1	3.745e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-3.891e-5	1	NC	1	NC	1
275		5	max	0	9	0	2	0	11	2.024e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-4.724e-5	1	NC	1	NC	1
277		6	max	0	9	0	2	0	2	3.028e-6	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-5.557e-5	1	NC	1	NC	1
279		7	max	0	9	0	2	0	10	2.09e-6	10	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.004	3	-.002	3	-6.39e-5	1	NC	1	NC	1
281		8	max	0	9	0	2	0	10	2.385e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-7.223e-5	1	NC	1	NC	1
283		9	max	0	9	0	2	0	10	2.68e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	3	-8.056e-5	1	NC	1	NC	1
285		10	max	0	9	.001	2	0	10	2.974e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-8.889e-5	1	NC	1	NC	1
287		11	max	0	9	.002	2	0	10	3.269e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	3	-9.722e-5	1	NC	1	NC	1
289		12	max	0	9	.002	2	0	10	3.564e-6	10	NC	1	NC	1
290			min	0	2	-.006	3	-.003	3	-1.056e-4	1	NC	1	NC	1
291		13	max	0	9	.003	2	0	10	3.859e-6	10	NC	1	NC	1
292			min	0	2	-.006	3	-.003	3	-1.175e-4	3	NC	1	NC	1
293		14	max	0	9	.003	2	0	10	4.154e-6	10	NC	1	NC	1
294			min	0	2	-.007	3	-.003	3	-1.347e-4	3	NC	1	NC	1
295		15	max	0	9	.004	2	0	10	4.448e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.002	3	-1.519e-4	3	NC	1	NC	1
297		16	max	0	9	.005	2	0	10	4.743e-6	10	NC	1	NC	1
298			min	0	2	-.007	3	-.002	3	-1.691e-4	3	9153.918	2	NC	1
299		17	max	0	9	.006	2	0	10	5.038e-6	10	NC	1	NC	1
300			min	0	2	-.007	3	-.002	3	-1.863e-4	3	7779.607	2	NC	1
301		18	max	0	9	.007	2	0	10	5.333e-6	10	NC	3	NC	1
302			min	0	2	-.007	3	-.002	3	-2.035e-4	3	6724.555	2	NC	1
303		19	max	0	9	.008	2	0	10	5.628e-6	10	NC	3	NC	1
304			min	0	2	-.007	3	-.002	1	-2.207e-4	3	5905.256	2	NC	1
305	M12	1	max	.002	1	.006	2	.002	1	2.352e-4	3	NC	1	NC	1
306			min	0	3	-.005	3	0	10	-5.621e-6	10	NC	1	NC	1
307		2	max	.002	1	.006	2	.001	1	2.352e-4	3	NC	1	NC	1
308			min	0	3	-.005	3	0	10	-5.621e-6	10	NC	1	NC	1
309		3	max	.001	1	.006	2	.001	1	2.352e-4	3	NC	1	NC	1
310			min	0	3	-.004	3	0	10	-5.621e-6	10	NC	1	NC	1
311		4	max	.001	1	.005	2	.001	1	2.352e-4	3	NC	1	NC	1
312			min	0	3	-.004	3	0	10	-5.621e-6	10	NC	1	NC	1
313		5	max	.001	1	.005	2	.001	1	2.352e-4	3	NC	1	NC	1
314			min	0	3	-.004	3	0	10	-5.621e-6	10	NC	1	NC	1
315		6	max	.001	1	.004	2	0	1	2.352e-4	3	NC	1	NC	1
316			min	0	3	-.004	3	0	10	-5.621e-6	10	NC	1	NC	1
317		7	max	.001	1	.004	2	0	1	2.352e-4	3	NC	1	NC	1
318			min	0	3	-.003	3	0	10	-5.621e-6	10	NC	1	NC	1
319		8	max	.001	1	.004	2	0	1	2.352e-4	3	NC	1	NC	1
320			min	0	3	-.003	3	0	10	-5.621e-6	10	NC	1	NC	1
321		9	max	0	1	.003	2	0	1	2.352e-4	3	NC	1	NC	1
322			min	0	3	-.003	3	0	10	-5.621e-6	10	NC	1	NC	1
323		10	max	0	1	.003	2	0	1	2.352e-4	3	NC	1	NC	1
324			min	0	3	-.002	3	0	10	-5.621e-6	10	NC	1	NC	1
325		11	max	0	1	.003	2	0	1	2.352e-4	3	NC	1	NC	1
326			min	0	3	-.002	3	0	10	-5.621e-6	10	NC	1	NC	1
327		12	max	0	1	.002	2	0	1	2.352e-4	3	NC	1	NC	1
328			min	0	3	-.002	3	0	10	-5.621e-6	10	NC	1	NC	1
329		13	max	0	1	.002	2	0	1	2.352e-4	3	NC	1	NC	1
330			min	0	3	-.002	3	0	10	-5.621e-6	10	NC	1	NC	1
331		14	max	0	1	.002	2	0	1	2.352e-4	3	NC	1	NC	1
332			min	0	3	-.001	3	0	10	-5.621e-6	10	NC	1	NC	1
333		15	max	0	1	.001	2	0	1	2.352e-4	3	NC	1	NC	1
334			min	0	3	-.001	3	0	10	-5.621e-6	10	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	2.352e-4	3	NC	1	NC	1
336			min	0	3	0	3	0	10	-5.621e-6	10	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	2.352e-4	3	NC	1	NC	1
338			min	0	3	0	3	0	10	-5.621e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.352e-4	3	NC	1	NC	1
340			min	0	3	0	3	0	10	-5.621e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.352e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-5.621e-6	10	NC	1	NC	1
343	M1	1	max	.005	3	.021	3	.002	3	6.458e-3	1	NC	1	NC	1
344			min	-.005	2	-.018	2	0	9	-8.51e-3	3	NC	1	NC	1
345		2	max	.005	3	.011	3	.002	3	3.161e-3	1	NC	4	NC	1
346			min	-.005	2	-.01	1	-.001	1	-4.179e-3	3	5008.146	3	NC	1
347		3	max	.005	3	.002	3	.001	3	7.251e-5	3	NC	4	NC	1
348			min	-.005	2	-.002	1	-.002	1	-7.585e-5	1	2600.548	3	NC	1
349		4	max	.005	3	.006	2	.001	3	7.022e-5	3	NC	4	NC	1
350			min	-.005	2	-.005	3	-.002	1	-5.902e-5	1	1862.214	3	NC	1
351		5	max	.005	3	.012	2	0	3	6.793e-5	3	NC	4	NC	1
352			min	-.005	2	-.011	3	-.002	1	-4.278e-5	9	1511.964	3	NC	1
353		6	max	.005	3	.017	2	0	3	6.564e-5	3	NC	4	NC	1
354			min	-.005	2	-.016	3	-.002	1	-3.011e-5	9	1292.473	2	NC	1
355		7	max	.005	3	.021	2	0	3	6.335e-5	3	NC	5	NC	1
356			min	-.006	2	-.02	3	-.002	1	-1.745e-5	9	1154.862	2	NC	1
357		8	max	.005	3	.024	2	0	3	6.106e-5	3	NC	5	NC	1
358			min	-.006	2	-.022	3	-.001	1	-4.778e-6	9	1069.143	2	NC	1
359		9	max	.005	3	.026	2	0	3	5.877e-5	3	NC	5	NC	1
360			min	-.006	2	-.023	3	0	1	-9.682e-7	10	1019.076	2	NC	1
361		10	max	.005	3	.026	2	0	3	5.648e-5	3	NC	5	NC	1
362			min	-.006	2	-.024	3	0	9	-1.546e-6	10	996.96	2	NC	1
363		11	max	.005	3	.026	2	0	3	5.874e-5	1	NC	5	NC	1
364			min	-.006	2	-.023	3	0	9	-2.124e-6	10	1000.092	2	NC	1
365		12	max	.005	3	.024	2	0	1	7.557e-5	1	NC	5	NC	1
366			min	-.006	2	-.021	3	0	10	-2.702e-6	10	1029.738	2	NC	1
367		13	max	.005	3	.022	2	.001	1	9.239e-5	1	NC	5	NC	1
368			min	-.006	2	-.018	3	0	10	-3.28e-6	10	1091.774	2	NC	1
369		14	max	.005	3	.017	2	.001	1	1.092e-4	1	NC	4	NC	1
370			min	-.006	2	-.014	3	0	10	-3.858e-6	10	1199.606	2	NC	1
371		15	max	.005	3	.012	2	.002	1	1.26e-4	1	NC	4	NC	1
372			min	-.006	2	-.01	3	0	10	-4.435e-6	10	1382.456	2	NC	1
373		16	max	.005	3	.005	2	.002	1	1.388e-4	1	NC	4	NC	1
374			min	-.006	2	-.004	3	0	10	-4.867e-6	10	1711.078	2	NC	1
375		17	max	.005	3	.002	3	.001	1	5.48e-5	1	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	-1.822e-6	10	2405.642	2	NC	1
377		18	max	.005	3	.009	3	0	1	4.258e-3	2	NC	4	NC	1
378			min	-.006	2	-.013	2	0	10	-2.179e-3	3	4647.749	2	NC	1
379		19	max	.005	3	.016	3	0	3	8.58e-3	2	NC	1	NC	1
380			min	-.006	2	-.023	2	0	9	-4.441e-3	3	NC	1	NC	1
381	M5	1	max	.014	3	.066	3	.002	3	3.442e-6	3	NC	1	NC	1
382			min	-.018	2	-.058	1	0	9	0	1	NC	1	NC	1
383		2	max	.014	3	.036	3	.003	3	7.677e-5	3	NC	4	NC	1
384			min	-.018	2	-.03	1	0	9	-1.152e-5	9	1584.62	3	NC	1
385		3	max	.014	3	.008	3	.004	3	1.487e-4	3	NC	5	NC	1
386			min	-.018	2	-.005	1	0	9	-2.285e-5	9	823.15	3	NC	1
387		4	max	.014	3	.018	2	.005	3	1.466e-4	3	NC	5	NC	1
388			min	-.018	2	-.016	3	0	9	-2.141e-5	9	590.018	3	NC	1
389		5	max	.014	3	.038	2	.005	3	1.445e-4	3	NC	5	NC	1
390			min	-.018	2	-.035	3	0	9	-1.998e-5	9	473.919	2	NC	1
391		6	max	.014	3	.054	2	.005	3	1.424e-4	3	NC	5	NC	1
392			min	-.018	2	-.05	3	0	9	-1.855e-5	9	403.759	2	NC	1
393		7	max	.014	3	.067	2	.006	3	1.403e-4	3	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.018	2	-.062	3	0	9	-1.712e-5	9	360.644	2	NC	1
395		8	max	.014	3	.077	2	.005	3	1.382e-4	3	NC	5	NC	1
396			min	-.018	2	-.069	3	0	9	-1.568e-5	9	333.776	2	NC	1
397		9	max	.014	3	.083	2	.005	3	1.361e-4	3	NC	5	NC	1
398			min	-.018	2	-.073	3	0	9	-1.425e-5	9	318.07	2	NC	1
399		10	max	.014	3	.085	2	.005	3	1.34e-4	3	NC	5	NC	1
400			min	-.018	2	-.074	3	0	9	-1.282e-5	9	311.112	2	NC	1
401		11	max	.014	3	.084	2	.005	3	1.318e-4	3	NC	5	NC	1
402			min	-.018	2	-.071	3	0	9	-1.139e-5	9	312.051	2	NC	1
403		12	max	.014	3	.078	2	.004	3	1.297e-4	3	NC	5	NC	1
404			min	-.018	2	-.065	3	0	9	-9.956e-6	9	321.281	2	NC	1
405		13	max	.014	3	.069	2	.004	3	1.276e-4	3	NC	5	NC	1
406			min	-.018	2	-.056	3	0	9	-8.524e-6	9	340.636	2	NC	1
407		14	max	.014	3	.056	2	.003	3	1.255e-4	3	NC	5	NC	1
408			min	-.018	2	-.045	3	0	9	-7.092e-6	9	374.302	2	NC	1
409		15	max	.014	3	.038	2	.002	3	1.234e-4	3	NC	5	NC	1
410			min	-.018	2	-.031	3	0	9	-5.66e-6	9	431.411	2	NC	1
411		16	max	.014	3	.017	2	.002	3	1.188e-4	3	NC	5	NC	1
412			min	-.018	2	-.014	3	0	9	-4.954e-6	9	534.081	2	NC	1
413		17	max	.014	3	.005	3	.001	3	5.348e-5	3	NC	5	NC	1
414			min	-.018	2	-.01	2	0	9	-2.153e-5	9	751.246	2	NC	1
415		18	max	.014	3	.027	3	0	3	2.615e-5	3	NC	4	NC	1
416			min	-.018	2	-.041	2	0	9	-1.105e-5	9	1451.896	2	NC	1
417		19	max	.014	3	.049	3	0	3	0	15	NC	1	NC	1
418			min	-.018	2	-.074	2	0	9	-4.699e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.021	3	.002	3	8.517e-3	3	NC	1	NC	1
420			min	-.005	2	-.018	2	0	9	-6.458e-3	1	NC	1	NC	1
421		2	max	.005	3	.011	3	0	3	4.236e-3	3	NC	4	NC	1
422			min	-.005	2	-.01	1	0	10	-3.178e-3	1	5010.706	3	NC	1
423		3	max	.005	3	.002	3	.001	1	4.189e-5	1	NC	4	NC	1
424			min	-.005	2	-.002	1	0	3	-2.323e-6	10	2601.913	3	NC	1
425		4	max	.005	3	.006	2	.002	1	2.668e-5	1	NC	4	NC	1
426			min	-.005	2	-.005	3	-.001	3	-1.753e-6	10	1863.19	3	NC	1
427		5	max	.005	3	.012	2	.002	1	1.366e-5	11	NC	4	NC	1
428			min	-.005	2	-.011	3	-.002	3	-2.972e-6	9	1512.722	3	NC	1
429		6	max	.005	3	.017	2	.001	1	6.152e-6	11	NC	4	NC	1
430			min	-.005	2	-.016	3	-.003	3	-1.333e-5	9	1292.72	2	NC	1
431		7	max	.005	3	.021	2	.001	1	-4.461e-8	10	NC	5	NC	1
432			min	-.005	2	-.02	3	-.003	3	-2.368e-5	9	1155.093	2	NC	1
433		8	max	.005	3	.024	2	0	1	5.25e-7	10	NC	5	NC	1
434			min	-.006	2	-.022	3	-.003	3	-3.417e-5	1	1069.366	2	9951.984	3
435		9	max	.005	3	.026	2	0	1	1.095e-6	10	NC	5	NC	1
436			min	-.006	2	-.024	3	-.004	3	-4.939e-5	1	1019.297	2	9780.469	3
437		10	max	.005	3	.026	2	0	11	1.664e-6	10	NC	5	NC	1
438			min	-.006	2	-.024	3	-.004	3	-6.46e-5	1	997.184	2	9872.473	3
439		11	max	.005	3	.026	2	0	10	2.234e-6	10	NC	5	NC	1
440			min	-.006	2	-.023	3	-.004	3	-7.981e-5	1	1000.323	2	NC	1
441		12	max	.005	3	.024	2	0	10	2.803e-6	10	NC	5	NC	1
442			min	-.006	2	-.021	3	-.003	3	-9.503e-5	1	1029.983	2	NC	1
443		13	max	.005	3	.022	2	0	10	3.373e-6	10	NC	5	NC	1
444			min	-.006	2	-.018	3	-.003	3	-1.102e-4	1	1092.04	2	NC	1
445		14	max	.005	3	.017	2	0	10	3.943e-6	10	NC	4	NC	1
446			min	-.006	2	-.014	3	-.003	3	-1.255e-4	1	1199.903	2	NC	1
447		15	max	.005	3	.012	2	0	10	4.512e-6	10	NC	4	NC	1
448			min	-.006	2	-.01	3	-.002	3	-1.407e-4	1	1382.801	2	NC	1
449		16	max	.005	3	.005	2	0	10	4.929e-6	10	NC	4	NC	1
450			min	-.006	2	-.004	3	-.002	1	-1.525e-4	1	1711.5	2	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.005	3	.002	3	0	10	1.706e-6	10	NC	4	NC	1
452		min	-.006	2	-.003	2	-.002	1	-8.454e-5	1	2406.189	2	NC	1
453	18	max	.005	3	.009	3	0	10	2.191e-3	3	NC	4	NC	1
454		min	-.006	2	-.013	2	-.001	1	-4.258e-3	2	4648.77	2	NC	1
455	19	max	.005	3	.016	3	0	3	4.44e-3	3	NC	1	NC	1
456		min	-.006	2	-.023	2	0	9	-8.58e-3	2	NC	1	NC	1
457	M13	1	max	0	.021	3	.005	3	3.639e-3	3	NC	1	NC	1
458		min	-.002	3	-.018	2	-.005	2	-3.25e-3	2	NC	1	NC	1
459	2	max	0	9	.065	3	.003	3	4.517e-3	3	NC	4	NC	1
460		min	-.002	3	-.051	1	-.005	2	-4.043e-3	2	2185.615	3	NC	1
461	3	max	0	9	.101	3	.005	9	5.394e-3	3	NC	5	NC	1
462		min	-.002	3	-.079	1	-.004	2	-4.835e-3	2	1188.895	3	NC	1
463	4	max	0	9	.126	3	.007	9	6.272e-3	3	NC	5	NC	2
464		min	-.002	3	-.099	1	-.005	2	-5.627e-3	2	908.76	3	9870.807	1
465	5	max	0	9	.137	3	.008	9	7.15e-3	3	NC	5	NC	2
466		min	-.002	3	-.107	1	-.006	2	-6.419e-3	2	825.887	3	9469.19	1
467	6	max	0	9	.133	3	.007	9	8.028e-3	3	NC	5	NC	1
468		min	-.002	3	-.105	1	-.008	2	-7.212e-3	2	852.093	3	NC	1
469	7	max	0	9	.118	3	.008	3	8.906e-3	3	NC	5	NC	1
470		min	-.002	3	-.095	1	-.011	2	-8.004e-3	2	984.579	3	NC	1
471	8	max	0	9	.097	3	.01	3	9.784e-3	3	NC	4	NC	1
472		min	-.002	3	-.079	1	-.014	2	-8.796e-3	2	1266.117	3	NC	1
473	9	max	0	9	.076	3	.012	3	1.066e-2	3	NC	4	NC	1
474		min	-.002	3	-.065	1	-.017	2	-9.588e-3	2	1740.443	3	8635.991	2
475	10	max	0	9	.066	3	.014	3	1.154e-2	3	NC	4	NC	1
476		min	-.002	3	-.058	1	-.018	2	-1.038e-2	2	2108.778	3	7819.577	2
477	11	max	0	9	.076	3	.015	3	1.066e-2	3	NC	4	NC	1
478		min	-.002	3	-.065	1	-.017	2	-9.588e-3	2	1740.443	3	8636.033	2
479	12	max	0	9	.097	3	.015	3	9.786e-3	3	NC	4	NC	1
480		min	-.002	3	-.079	1	-.014	2	-8.796e-3	2	1266.116	3	8955.289	3
481	13	max	0	9	.118	3	.015	3	8.909e-3	3	NC	5	NC	1
482		min	-.002	3	-.095	1	-.011	2	-8.004e-3	2	984.579	3	9367.504	3
483	14	max	0	9	.134	3	.014	3	8.032e-3	3	NC	5	NC	1
484		min	-.002	3	-.105	1	-.008	2	-7.212e-3	2	852.093	3	NC	1
485	15	max	0	9	.137	3	.012	3	7.156e-3	3	NC	5	NC	2
486		min	-.002	3	-.107	1	-.006	2	-6.419e-3	2	825.887	3	9470.174	1
487	16	max	0	9	.127	3	.01	3	6.279e-3	3	NC	5	NC	2
488		min	-.002	3	-.099	1	-.005	2	-5.627e-3	2	908.76	3	9877.023	1
489	17	max	0	9	.102	3	.008	3	5.402e-3	3	NC	5	NC	1
490		min	-.002	3	-.079	1	-.004	2	-4.835e-3	2	1188.895	3	NC	1
491	18	max	0	9	.065	3	.006	3	4.525e-3	3	NC	4	NC	1
492		min	-.002	3	-.051	1	-.005	2	-4.043e-3	2	2185.615	3	NC	1
493	19	max	0	9	.021	3	.005	3	3.648e-3	3	NC	1	NC	1
494		min	-.002	3	-.018	2	-.005	2	-3.251e-3	2	NC	1	NC	1
495	M16	1	max	0	.016	3	.005	3	3.923e-3	2	NC	1	NC	1
496		min	0	3	-.023	2	-.006	2	-2.746e-3	3	NC	1	NC	1
497	2	max	0	9	.039	3	.006	3	4.883e-3	2	NC	4	NC	1
498		min	0	3	-.067	2	-.005	2	-3.389e-3	3	2167.302	2	NC	1
499	3	max	0	9	.058	3	.008	3	5.843e-3	2	NC	5	NC	1
500		min	0	3	-.105	2	-.004	2	-4.032e-3	3	1177.026	2	NC	1
501	4	max	0	9	.072	3	.01	3	6.803e-3	2	NC	5	NC	1
502		min	0	3	-.13	2	-.005	2	-4.675e-3	3	897.12	2	NC	1
503	5	max	0	9	.079	3	.012	3	7.763e-3	2	NC	5	NC	2
504		min	0	3	-.141	2	-.006	2	-5.317e-3	3	811.584	2	9678.73	1
505	6	max	0	9	.078	3	.013	3	8.724e-3	2	NC	5	NC	1
506		min	0	3	-.139	2	-.008	2	-5.96e-3	3	831.269	2	NC	1
507	7	max	0	9	.072	3	.014	3	9.684e-3	2	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.124	2	-.011	2	-6.603e-3	3	949.21	2	NC	1
509	8	max	0	9	.063	3	.015	3	1.064e-2	2	NC	4	NC	1
510		min	0	3	-.103	2	-.014	2	-7.246e-3	3	1196.841	2	9699.877	3
511	9	max	0	9	.054	3	.014	3	1.16e-2	2	NC	4	NC	1
512		min	0	3	-.083	2	-.017	2	-7.889e-3	3	1596.753	2	8455.24	2
513	10	max	0	9	.049	3	.014	3	1.256e-2	2	NC	4	NC	3
514		min	0	3	-.074	2	-.018	2	-8.532e-3	3	1892.646	2	7667.481	2
515	11	max	0	9	.054	3	.013	3	1.16e-2	2	NC	4	NC	1
516		min	0	3	-.083	2	-.017	2	-7.888e-3	3	1596.753	2	8455.269	2
517	12	max	0	9	.063	3	.012	3	1.064e-2	2	NC	4	NC	1
518		min	0	3	-.103	2	-.014	2	-7.244e-3	3	1196.841	2	NC	1
519	13	max	0	9	.072	3	.011	3	9.684e-3	2	NC	5	NC	1
520		min	0	3	-.124	2	-.011	2	-6.601e-3	3	949.21	2	NC	1
521	14	max	0	9	.078	3	.01	3	8.724e-3	2	NC	5	NC	1
522		min	0	3	-.139	2	-.008	2	-5.957e-3	3	831.269	2	NC	1
523	15	max	0	9	.079	3	.009	3	7.764e-3	2	NC	5	NC	2
524		min	0	3	-.141	2	-.006	2	-5.313e-3	3	811.584	2	9691.362	1
525	16	max	0	9	.072	3	.007	3	6.804e-3	2	NC	5	NC	1
526		min	0	3	-.13	2	-.005	2	-4.669e-3	3	897.12	2	NC	1
527	17	max	0	9	.058	3	.006	3	5.844e-3	2	NC	5	NC	1
528		min	0	3	-.105	2	-.004	2	-4.026e-3	3	1177.026	2	NC	1
529	18	max	0	9	.039	3	.005	3	4.884e-3	2	NC	4	NC	1
530		min	0	3	-.067	2	-.005	2	-3.382e-3	3	2167.302	2	NC	1
531	19	max	0	9	.016	3	.005	3	3.924e-3	2	NC	1	NC	1
532		min	0	3	-.023	2	-.006	2	-2.738e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.236e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.425e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.622e-4	3	NC	1	NC	1
536		min	0	1	-.002	4	0	3	-4.771e-4	2	NC	1	NC	1
537	3	max	0	3	0	15	.002	1	1.201e-3	3	NC	1	NC	1
538		min	0	1	-.004	4	-.003	3	-9.1e-4	2	NC	1	NC	1
539	4	max	0	3	-.001	15	.005	1	1.64e-3	3	NC	1	NC	4
540		min	0	1	-.006	4	-.006	3	-1.343e-3	2	9528.629	4	6575.891	3
541	5	max	0	3	-.002	15	.008	1	2.078e-3	3	NC	3	NC	4
542		min	0	1	-.008	4	-.01	3	-1.776e-3	2	7435.286	4	4280.039	3
543	6	max	0	3	-.002	15	.012	1	2.517e-3	3	NC	3	NC	4
544		min	0	1	-.009	4	-.014	3	-2.209e-3	2	6257.579	4	3098.591	3
545	7	max	0	3	-.002	15	.016	1	2.956e-3	3	NC	5	NC	4
546		min	0	1	-.01	4	-.018	3	-2.641e-3	2	5549.344	4	2412.153	3
547	8	max	0	3	-.003	15	.019	1	3.394e-3	3	NC	5	NC	4
548		min	-.001	1	-.011	4	-.022	3	-3.074e-3	2	5124.296	4	1982.531	3
549	9	max	0	3	-.003	15	.023	1	3.833e-3	3	NC	5	NC	4
550		min	-.001	1	-.012	4	-.026	3	-3.507e-3	2	4895.51	4	1702.17	3
551	10	max	0	3	-.003	15	.025	1	4.272e-3	3	NC	5	NC	4
552		min	-.001	1	-.012	4	-.029	3	-3.94e-3	2	4823.133	4	1517.337	3
553	11	max	0	3	-.003	15	.027	1	4.71e-3	3	NC	5	NC	4
554		min	-.002	1	-.012	4	-.032	3	-4.373e-3	2	4895.51	4	1399.893	3
555	12	max	0	3	-.003	15	.028	1	5.149e-3	3	NC	5	NC	5
556		min	-.002	1	-.011	4	-.033	3	-4.806e-3	2	5124.296	4	1335.824	3
557	13	max	0	3	-.002	12	.028	1	5.587e-3	3	NC	5	NC	5
558		min	-.002	1	-.011	4	-.032	3	-5.239e-3	2	5549.344	4	1320.871	3
559	14	max	0	3	-.002	12	.026	1	6.026e-3	3	NC	3	NC	4
560		min	-.002	1	-.009	4	-.03	3	-5.672e-3	2	6257.579	4	1360.533	3
561	15	max	0	3	-.001	12	.023	1	6.465e-3	3	NC	3	NC	4
562		min	-.002	1	-.008	4	-.026	3	-6.104e-3	2	7435.286	4	1475.72	3
563	16	max	0	3	0	3	.017	1	6.903e-3	3	NC	1	NC	4
564		min	-.002	1	-.006	4	-.02	3	-6.537e-3	2	9528.629	4	1723.524	3

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	.001	3	.009	1	7.342e-3	3	NC	1	NC	4
566			min	-0.003	1	-0.005	4	-.011	3	-6.97e-3	2	NC	1	2283.302	3
567		18	max	0	3	.003	3	.001	9	7.781e-3	3	NC	1	NC	4
568			min	-0.003	1	-.003	14	-.005	2	-7.403e-3	2	NC	1	4062.632	3
569		19	max	.001	3	.004	3	.016	3	8.219e-3	3	NC	1	NC	1
570			min	-.003	1	-.001	9	-.018	2	-7.836e-3	2	NC	1	NC	1
571	M16A	1	max	0	2	0	3	.005	3	2.433e-3	3	NC	1	NC	1
572			min	-.001	3	0	1	-.006	2	-2.429e-3	2	NC	1	NC	1
573		2	max	0	2	0	12	0	9	2.329e-3	3	NC	1	NC	1
574			min	0	3	-.002	1	-.001	2	-2.315e-3	2	NC	1	NC	1
575		3	max	0	2	-.001	15	.004	1	2.224e-3	3	NC	1	NC	4
576			min	0	3	-.004	4	-.004	3	-2.2e-3	2	NC	1	6413.176	3
577		4	max	0	2	-.001	15	.006	1	2.119e-3	3	NC	1	NC	4
578			min	0	3	-.006	4	-.007	3	-2.086e-3	2	9528.629	4	4868.898	3
579		5	max	0	2	-.002	15	.008	1	2.015e-3	3	NC	3	NC	4
580			min	0	3	-.008	4	-.01	3	-1.972e-3	2	7435.286	4	4196.295	3
581		6	max	0	2	-.002	15	.009	1	1.91e-3	3	NC	3	NC	4
582			min	0	3	-.009	4	-.011	3	-1.858e-3	2	6257.579	4	3898.005	3
583		7	max	0	2	-.002	15	.01	1	1.806e-3	3	NC	5	NC	4
584			min	0	3	-.01	4	-.011	3	-1.744e-3	2	5549.344	4	3817.59	3
585		8	max	0	2	-.003	15	.01	1	1.701e-3	3	NC	5	NC	4
586			min	0	3	-.011	4	-.011	3	-1.63e-3	2	5124.296	4	3900.628	3
587		9	max	0	2	-.003	15	.009	1	1.597e-3	3	NC	5	NC	4
588			min	0	3	-.012	4	-.011	3	-1.515e-3	2	4895.51	4	4137.971	3
589		10	max	0	2	-.003	15	.008	1	1.492e-3	3	NC	5	NC	4
590			min	0	3	-.012	4	-.01	3	-1.401e-3	2	4823.133	4	4552.004	3
591		11	max	0	2	-.003	15	.007	1	1.387e-3	3	NC	5	NC	4
592			min	0	3	-.012	4	-.009	3	-1.287e-3	2	4895.51	4	5200.795	3
593		12	max	0	2	-.003	15	.006	1	1.283e-3	3	NC	5	NC	4
594			min	0	3	-.011	4	-.007	3	-1.173e-3	2	5124.296	4	6199.596	3
595		13	max	0	2	-.002	15	.004	1	1.178e-3	3	NC	5	NC	2
596			min	0	3	-.01	4	-.006	3	-1.059e-3	2	5549.344	4	7775.866	3
597		14	max	0	2	-.002	15	.003	1	1.074e-3	3	NC	3	NC	1
598			min	0	3	-.009	4	-.004	3	-9.446e-4	2	6257.579	4	NC	1
599		15	max	0	2	-.002	15	.002	1	9.691e-4	3	NC	3	NC	1
600			min	0	3	-.008	4	-.003	3	-8.304e-4	2	7435.286	4	NC	1
601		16	max	0	2	-.001	15	0	1	8.645e-4	3	NC	1	NC	1
602			min	0	3	-.006	4	-.001	3	-7.163e-4	2	9528.629	4	NC	1
603		17	max	0	2	0	15	0	4	7.599e-4	3	NC	1	NC	1
604			min	0	3	-.004	4	0	3	-6.021e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	4	6.554e-4	3	NC	1	NC	1
606			min	0	3	-.002	4	0	2	-4.88e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	5.508e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.738e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.