

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

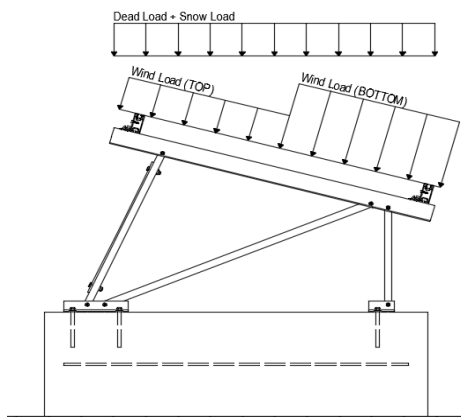
### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

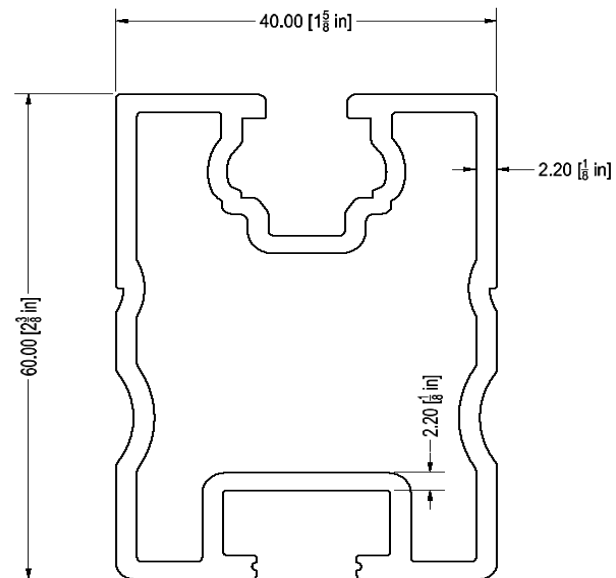
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

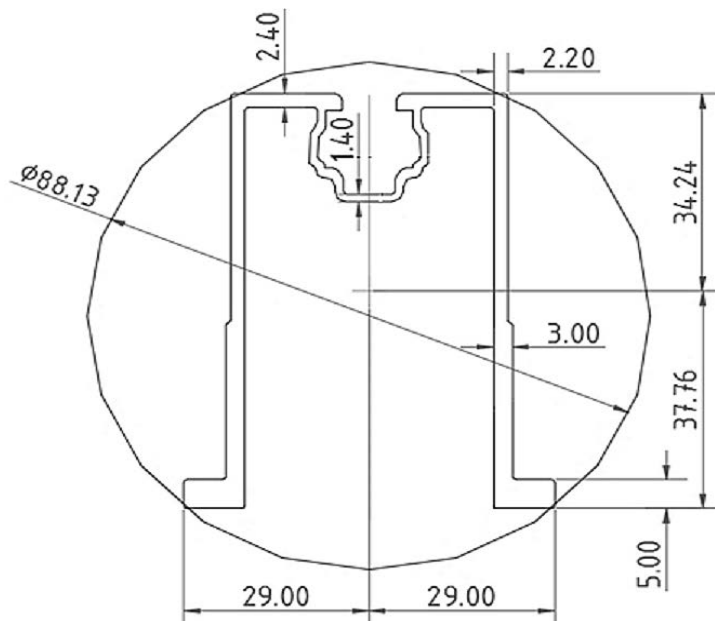
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	45 in
$\Phi F_{ty}$ STRONG-AXIS =	29.87 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.337 k-ft
$M_z$ =	0.034 k-ft
$M_{y \text{ allowable}}$ =	1.271 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>30%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

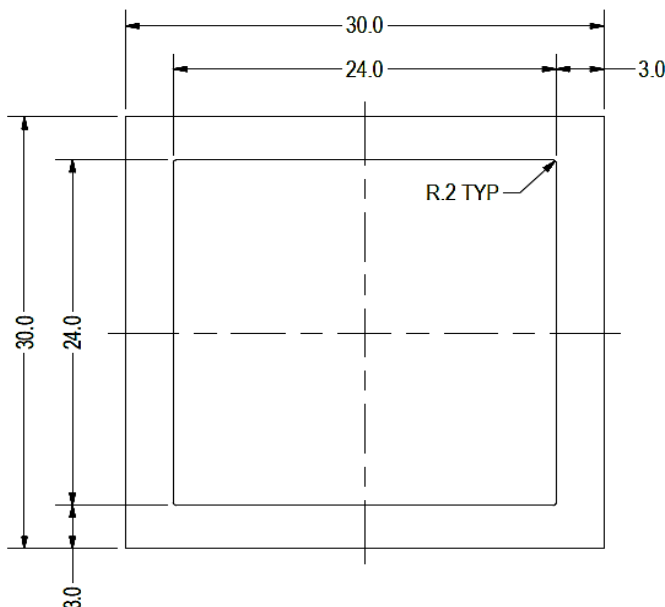
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.76 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.512 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.267 k
$M_{y \text{ allowable}}$ =	1.460 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>37%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

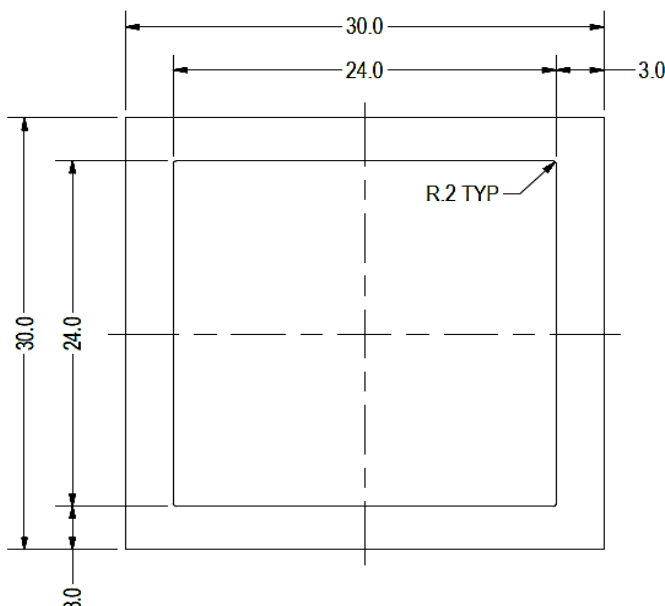
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.025 k-ft
$P_n$ =	0.132 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.571 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>15%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.604 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>12%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.169 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

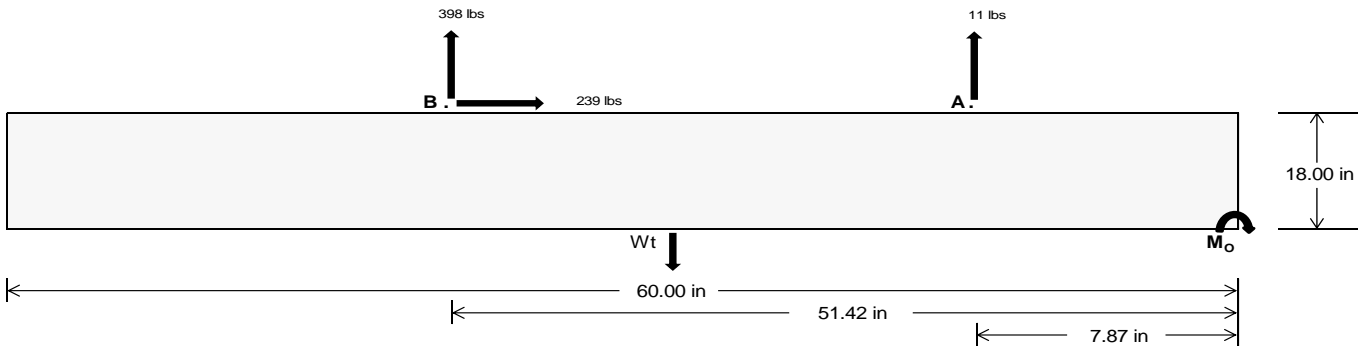
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>49.61</b>	<b>1659.31</b>	k
Compressive Load =	<b>927.67</b>	<b>1086.20</b>	k
Lateral Load =	<b>20.31</b>	<b>992.63</b>	k
Moment (Weak Axis) =	<b>0.03</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 24871.6$  in-lbs  
Resisting Force Required = 829.05 lbs  
S.F. = 1.67  
Weight Required = 1381.76 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 238.57 lbs  
Friction = 0.4  
Weight Required = 596.43 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 238.57 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

		Ballast Width			
		21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$		1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	303 lbs	303 lbs	303 lbs	303 lbs	370 lbs	370 lbs	370 lbs	370 lbs	477 lbs	477 lbs	477 lbs	477 lbs	-22 lbs	-22 lbs	-22 lbs	-22 lbs
$F_B$	202 lbs	202 lbs	202 lbs	202 lbs	481 lbs	481 lbs	481 lbs	481 lbs	493 lbs	493 lbs	493 lbs	493 lbs	-797 lbs	-797 lbs	-797 lbs	-797 lbs
$F_V$	25 lbs	25 lbs	25 lbs	25 lbs	426 lbs	426 lbs	426 lbs	426 lbs	337 lbs	337 lbs	337 lbs	337 lbs	-477 lbs	-477 lbs	-477 lbs	-477 lbs
$P_{total}$	2409 lbs	2499 lbs	2590 lbs	2681 lbs	2754 lbs	2845 lbs	2936 lbs	3026 lbs	2873 lbs	2964 lbs	3054 lbs	3145 lbs	323 lbs	377 lbs	431 lbs	486 lbs
$M$	236 lbs-ft	236 lbs-ft	236 lbs-ft	236 lbs-ft	464 lbs-ft	464 lbs-ft	464 lbs-ft	464 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	505 lbs-ft	665 lbs-ft	665 lbs-ft	665 lbs-ft	665 lbs-ft
$e$	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.06 ft	1.77 ft	1.54 ft	1.37 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	243.0 psf	241.8 psf	240.7 psf	239.8 psf	251.2 psf	249.6 psf	248.2 psf	246.9 psf	259.1 psf	257.2 psf	255.5 psf	253.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	307.6 psf	303.5 psf	299.8 psf	296.4 psf	378.4 psf	371.1 psf	364.4 psf	358.3 psf	397.6 psf	389.4 psf	381.9 psf	375.1 psf	281.4 psf	186.6 psf	156.8 psf	143.3 psf

Maximum Bearing Pressure = 398 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

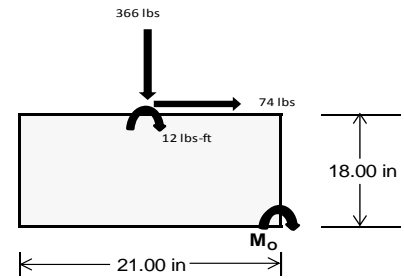
### Overturning Check

$M_o = 197.2 \text{ ft-lbs}$   
 Resisting Force Required = 225.37 lbs  
 S.F. = 1.67  
 Weight Required = 375.62 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	116 lbs	46 lbs	54 lbs	203 lbs	366 lbs	155 lbs	81 lbs	-38 lbs	20 lbs
$F_v$	12 lbs	98 lbs	12 lbs	9 lbs	74 lbs	9 lbs	12 lbs	98 lbs	12 lbs
$P_{total}$	2472 lbs	2402 lbs	2410 lbs	2446 lbs	2609 lbs	2398 lbs	770 lbs	651 lbs	709 lbs
$M$	33 lbs-ft	163 lbs-ft	34 lbs-ft	24 lbs-ft	123 lbs-ft	27 lbs-ft	33 lbs-ft	163 lbs-ft	34 lbs-ft
$e$	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.25 ft	0.05 ft
$L/6$	0.29 ft	1.61 ft	1.72 ft	1.73 ft	1.66 ft	1.73 ft	1.66 ft	1.25 ft	1.65 ft
$f_{min}$	269.7 sqft	210.5 sqft	262.1 sqft	270.1 sqft	250.0 sqft	263.6 sqft	75.0 sqft	10.4 sqft	67.8 sqft
$f_{max}$	295.5 psf	338.6 psf	288.6 psf	288.8 psf	346.3 psf	284.4 psf	101.0 psf	138.4 psf	94.2 psf



Maximum Bearing Pressure = 346 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.784 k
Allowable Uplift =	1.214 k
Utilization =	<u>65%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.052 k
Allowable Uplift =	1.116 k
Utilization =	<u>94%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.714 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

#### Diagonal Strut

Maximum Axial Load =	0.571 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>



#### Rear Strut

Maximum Axial Load =	1.042 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

#### Bracing

Maximum Axial Load =	0.169 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.05 in
	<u>0.05 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$117.177$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$121.682$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.271 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &21.4323 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	196.077	2	-.016	15	.085	1	0	10	0	4	0	15
30		min	-349.282	3	-.072	3	-.571	5	0	4	0	3	0	6
31	16	max	196.203	2	-.028	15	.085	1	0	10	0	4	0	15
32		min	-349.188	3	-.114	4	-.685	5	0	4	0	3	0	6
33	17	max	196.329	2	-.04	15	.085	1	0	10	0	4	0	15
34		min	-349.093	3	-.165	4	-.8	5	0	4	0	3	0	6
35	18	max	196.455	2	-.052	15	.085	1	0	10	0	14	0	15
36		min	-348.999	3	-.216	4	-.914	5	0	4	0	3	0	6
37	19	max	196.581	2	-.064	15	.085	1	0	10	0	1	0	15
38		min	-348.905	3	-.267	4	-1.028	5	0	4	0	3	0	6
39	M3	1	max	183.61	2	1.757	.006	10	0	5	0	4	0	6
40		min	-173.449	3	.412	15	-1.324	4	0	1	0	10	0	15
41	2	max	183.541	2	1.58	6	-.006	10	0	5	0	1	0	2
42		min	-173.501	3	.37	15	-1.19	4	0	1	0	10	0	12
43	3	max	183.471	2	1.403	6	.006	10	0	5	0	1	0	2
44		min	-173.553	3	.329	15	-1.057	4	0	1	0	5	0	3
45	4	max	183.402	2	1.226	6	.006	10	0	5	0	1	0	15
46		min	-173.605	3	.287	15	-.923	4	0	1	0	5	0	4
47	5	max	183.333	2	1.049	6	.006	10	0	5	0	1	0	15
48		min	-173.657	3	.246	15	-.789	4	0	1	0	5	0	4
49	6	max	183.263	2	.873	6	.006	10	0	5	0	1	0	15
50		min	-173.709	3	.204	15	-.656	4	0	1	0	5	0	4
51	7	max	183.194	2	.696	6	-.006	10	0	5	0	1	0	15
52		min	-173.761	3	.163	15	-.522	4	0	1	0	5	0	4
53	8	max	183.125	2	.519	6	.006	10	0	5	0	1	0	15
54		min	-173.813	3	.121	15	-.388	4	0	1	0	5	-.001	4
55	9	max	183.055	2	.342	6	.006	10	0	5	0	1	0	15
56		min	-173.865	3	.08	15	-.255	4	0	1	0	5	-.001	4
57	10	max	182.986	2	.165	6	.006	10	0	5	0	1	0	15
58		min	-173.917	3	.038	15	-.122	1	0	1	0	5	-.001	4
59	11	max	182.917	2	.018	2	.043	5	0	5	0	1	0	15
60		min	-173.969	3	-.037	3	-.122	1	0	1	0	5	-.001	4
61	12	max	182.847	2	-.045	15	.176	5	0	5	0	1	0	15
62		min	-174.021	3	-.189	4	-.122	1	0	1	0	5	-.001	4
63	13	max	182.778	2	-.087	15	.31	5	0	5	0	1	0	15
64		min	-174.073	3	-.365	4	-.122	1	0	1	0	5	-.001	4
65	14	max	182.709	2	-.128	15	.444	5	0	5	0	1	0	15
66		min	-174.125	3	-.542	4	-.122	1	0	1	0	5	-.001	4
67	15	max	182.639	2	-.17	15	.577	5	0	5	0	9	0	15
68		min	-174.177	3	-.719	4	-.122	1	0	1	0	5	0	4
69	16	max	182.57	2	-.211	15	.711	5	0	5	0	9	0	15
70		min	-174.229	3	-.896	4	-.122	1	0	1	0	5	0	4
71	17	max	182.501	2	-.253	15	.845	5	0	5	0	10	0	15
72		min	-174.281	3	-1.073	4	-.122	1	0	1	0	4	0	4
73	18	max	182.431	2	-.295	15	.978	5	0	5	0	10	0	15
74		min	-174.333	3	-1.25	4	-.122	1	0	1	0	4	0	4
75	19	max	182.362	2	-.336	15	1.112	5	0	5	0	5	0	1
76		min	-174.385	3	-1.426	4	-.122	1	0	1	0	1	0	1
77	M4	1	max	254.877	1	0	.037	10	0	1	0	5	0	1
78		min	2.806	12	0	1	-14.46	4	0	1	0	2	0	1
79	2	max	254.942	1	0	1	.037	10	0	1	0	10	0	1
80		min	2.838	12	0	1	-14.516	4	0	1	-.001	4	0	1
81	3	max	255.007	1	0	1	.037	10	0	1	0	10	0	1
82		min	2.87	12	0	1	-14.572	4	0	1	-.003	4	0	1
83	4	max	255.072	1	0	1	.037	10	0	1	0	10	0	1
84		min	2.903	12	0	1	-14.628	4	0	1	-.004	4	0	1
85	5	max	255.136	1	0	1	.037	10	0	1	0	10	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	2.935	12	0	1	-14.684	4	0	1	-.005	4	0	1
87		6	max	255.201	1	0	1	.037	10	0	1	0	10	0	1
88			min	2.967	12	0	1	-14.74	4	0	1	-.007	4	0	1
89		7	max	255.266	1	0	1	.037	10	0	1	0	10	0	1
90			min	3	12	0	1	-14.796	4	0	1	-.008	4	0	1
91		8	max	255.33	1	0	1	.037	10	0	1	0	10	0	1
92			min	3.032	12	0	1	-14.852	4	0	1	-.009	4	0	1
93		9	max	255.395	1	0	1	.037	10	0	1	0	10	0	1
94			min	3.064	12	0	1	-14.909	4	0	1	-.01	4	0	1
95		10	max	255.46	1	0	1	.037	10	0	1	0	10	0	1
96			min	3.097	12	0	1	-14.965	4	0	1	-.012	4	0	1
97		11	max	255.525	1	0	1	.037	10	0	1	0	10	0	1
98			min	3.129	12	0	1	-15.021	4	0	1	-.013	4	0	1
99		12	max	255.589	1	0	1	.037	10	0	1	0	10	0	1
100			min	3.161	12	0	1	-15.077	4	0	1	-.015	4	0	1
101		13	max	255.654	1	0	1	.037	10	0	1	0	10	0	1
102			min	3.194	12	0	1	-15.133	4	0	1	-.016	4	0	1
103		14	max	255.719	1	0	1	.037	10	0	1	0	10	0	1
104			min	3.226	12	0	1	-15.189	4	0	1	-.017	4	0	1
105		15	max	255.783	1	0	1	.037	10	0	1	0	10	0	1
106			min	3.258	12	0	1	-15.245	4	0	1	-.019	4	0	1
107		16	max	255.848	1	0	1	.037	10	0	1	0	10	0	1
108			min	3.291	12	0	1	-15.301	4	0	1	-.02	4	0	1
109		17	max	255.913	1	0	1	.037	10	0	1	0	10	0	1
110			min	3.323	12	0	1	-15.357	4	0	1	-.021	4	0	1
111		18	max	255.977	1	0	1	.037	10	0	1	0	10	0	1
112			min	3.356	12	0	1	-15.413	4	0	1	-.023	4	0	1
113		19	max	256.042	1	0	1	.037	10	0	1	0	10	0	1
114			min	3.388	12	0	1	-15.469	4	0	1	-.024	4	0	1
115	M6	1	max	601.557	2	.638	6	1.006	4	0	3	0	3	0	1
116			min	-1042.315	3	.142	15	-.277	3	0	5	0	2	0	1
117		2	max	601.683	2	.587	6	.892	4	0	3	0	3	0	15
118			min	-1042.221	3	.13	15	-.277	3	0	5	0	2	0	6
119		3	max	601.809	2	.535	6	.778	4	0	3	0	4	0	15
120			min	-1042.126	3	.117	15	-.277	3	0	5	0	2	0	6
121		4	max	601.935	2	.484	6	.663	4	0	3	0	4	0	15
122			min	-1042.032	3	.105	15	-.277	3	0	5	0	2	0	6
123		5	max	602.061	2	.442	2	.549	4	0	3	0	4	0	15
124			min	-1041.937	3	.093	15	-.277	3	0	5	0	2	0	6
125		6	max	602.187	2	.402	2	.434	4	0	3	0	4	0	15
126			min	-1041.843	3	.081	15	-.277	3	0	5	0	2	0	6
127		7	max	602.312	2	.363	2	.32	4	0	3	0	4	0	15
128			min	-1041.749	3	.067	12	-.277	3	0	5	0	3	0	6
129		8	max	602.438	2	.323	2	.206	4	0	3	0	4	0	15
130			min	-1041.654	3	.047	12	-.277	3	0	5	0	3	0	2
131		9	max	602.564	2	.283	2	.091	4	0	3	0	4	0	15
132			min	-1041.56	3	.027	12	-.277	3	0	5	0	3	0	2
133		10	max	602.69	2	.243	2	.017	9	0	3	0	4	0	15
134			min	-1041.465	3	.003	3	-.277	3	0	5	0	3	0	2
135		11	max	602.816	2	.203	2	.017	9	0	3	0	4	0	15
136			min	-1041.371	3	-.027	3	-.277	3	0	5	0	3	0	2
137		12	max	602.942	2	.163	2	.017	9	0	3	0	4	0	15
138			min	-1041.277	3	-.057	3	-.277	3	0	5	0	3	0	2
139		13	max	603.068	2	.123	2	.017	9	0	3	0	4	0	12
140			min	-1041.182	3	-.086	3	-.374	5	0	5	0	3	0	2
141		14	max	603.194	2	.084	2	.017	9	0	3	0	4	0	12
142			min	-1041.088	3	-.116	3	-.488	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	603.319	2	.044	2	.017	9	0	3	0	4	0	12
144		min	-1040.993	3	-.146	3	-.602	5	0	5	0	3	0	2
145	16	max	603.445	2	.004	2	.017	9	0	3	0	4	0	12
146		min	-1040.899	3	-.176	3	-.717	5	0	5	0	3	0	2
147	17	max	603.571	2	-.036	2	.017	9	0	3	0	4	0	12
148		min	-1040.805	3	-.206	3	-.831	5	0	5	0	3	0	2
149	18	max	603.697	2	-.063	15	.017	9	0	3	0	4	0	12
150		min	-1040.71	3	-.236	3	-.946	5	0	5	0	3	0	2
151	19	max	603.823	2	-.075	15	.017	9	0	3	0	9	0	3
152		min	-1040.616	3	-.284	4	-1.06	5	0	5	0	3	0	2
153	M7	1	max	571.344	2	1.776	.038	3	0	9	0	4	0	2
154		min	-475.711	3	.424	15	-1.323	4	0	3	0	3	0	3
155	2	max	571.274	2	1.599	4	.038	3	0	9	0	4	0	2
156		min	-475.763	3	.383	15	-1.189	4	0	3	0	3	0	3
157	3	max	571.205	2	1.422	4	.038	3	0	9	0	1	0	2
158		min	-475.815	3	.341	15	-1.056	4	0	3	0	3	0	3
159	4	max	571.136	2	1.245	4	.038	3	0	9	0	1	0	2
160		min	-475.867	3	.299	15	-.922	4	0	3	0	3	0	3
161	5	max	571.066	2	1.069	4	.038	3	0	9	0	1	0	15
162		min	-475.919	3	.258	15	-.788	4	0	3	0	5	0	3
163	6	max	570.997	2	.892	4	.038	3	0	9	0	1	0	15
164		min	-475.971	3	.216	15	-.655	4	0	3	0	5	0	6
165	7	max	570.928	2	.715	4	.038	3	0	9	0	1	0	15
166		min	-476.023	3	.175	15	-.521	4	0	3	0	5	0	6
167	8	max	570.858	2	.538	4	.038	3	0	9	0	1	0	15
168		min	-476.075	3	.133	15	-.387	4	0	3	0	5	-.001	6
169	9	max	570.789	2	.361	4	.038	3	0	9	0	1	0	15
170		min	-476.127	3	.079	12	-.254	4	0	3	0	5	-.001	6
171	10	max	570.72	2	.208	2	.038	3	0	9	0	1	0	15
172		min	-476.179	3	.01	3	-.12	4	0	3	0	5	-.001	6
173	11	max	570.65	2	.07	2	.038	3	0	9	0	1	0	15
174		min	-476.231	3	-.094	3	-.015	1	0	3	-.001	5	-.001	6
175	12	max	570.581	2	-.033	15	.148	5	0	9	0	1	0	15
176		min	-476.283	3	-.197	3	-.015	1	0	3	0	5	-.001	6
177	13	max	570.512	2	-.075	15	.282	5	0	9	0	1	0	15
178		min	-476.335	3	-.347	6	-.015	1	0	3	0	5	-.001	6
179	14	max	570.442	2	-.116	15	.416	5	0	9	0	1	0	15
180		min	-476.387	3	-.523	6	-.015	1	0	3	0	5	-.001	6
181	15	max	570.373	2	-.158	15	.549	5	0	9	0	1	0	15
182		min	-476.439	3	-.7	6	-.015	1	0	3	0	5	0	6
183	16	max	570.304	2	-.199	15	.683	5	0	9	0	1	0	15
184		min	-476.491	3	-.877	6	-.015	1	0	3	0	5	0	6
185	17	max	570.235	2	-.241	15	.817	5	0	9	0	9	0	15
186		min	-476.543	3	-1.054	6	-.015	1	0	3	0	5	0	6
187	18	max	570.165	2	-.283	15	.951	5	0	9	0	9	0	15
188		min	-476.595	3	-1.231	6	-.015	1	0	3	0	3	0	6
189	19	max	570.096	2	-.324	15	1.084	5	0	9	0	9	0	1
190		min	-476.647	3	-1.408	6	-.015	1	0	3	0	3	0	1
191	M8	1	max	712.43	1	0	.12	9	0	1	0	4	0	1
192		min	-39.034	3	0	1	-14.727	4	0	1	0	3	0	1
193	2	max	712.495	1	0	1	.12	9	0	1	0	9	0	1
194		min	-38.985	3	0	1	-14.783	4	0	1	-.001	4	0	1
195	3	max	712.56	1	0	1	.12	9	0	1	0	9	0	1
196		min	-38.937	3	0	1	-14.839	4	0	1	-.003	4	0	1
197	4	max	712.625	1	0	1	.12	9	0	1	0	9	0	1
198		min	-38.888	3	0	1	-14.895	4	0	1	-.004	4	0	1
199	5	max	712.689	1	0	1	.12	9	0	1	0	9	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-38.84	3	0	1	-14.951	4	0	1	-.005	4	0	1
201		6	max	712.754	1	0	1	.12	9	0	1	0	9	0	1
202			min	-38.791	3	0	1	-15.007	4	0	1	-.007	4	0	1
203		7	max	712.819	1	0	1	.12	9	0	1	0	9	0	1
204			min	-38.743	3	0	1	-15.063	4	0	1	-.008	4	0	1
205		8	max	712.883	1	0	1	.12	9	0	1	0	9	0	1
206			min	-38.694	3	0	1	-15.119	4	0	1	-.009	4	0	1
207		9	max	712.948	1	0	1	.12	9	0	1	0	9	0	1
208			min	-38.646	3	0	1	-15.175	4	0	1	-.011	4	0	1
209		10	max	713.013	1	0	1	.12	9	0	1	0	9	0	1
210			min	-38.597	3	0	1	-15.232	4	0	1	-.012	4	0	1
211		11	max	713.077	1	0	1	.12	9	0	1	0	9	0	1
212			min	-38.549	3	0	1	-15.288	4	0	1	-.013	4	0	1
213		12	max	713.142	1	0	1	.12	9	0	1	0	9	0	1
214			min	-38.5	3	0	1	-15.344	4	0	1	-.015	4	0	1
215		13	max	713.207	1	0	1	.12	9	0	1	0	9	0	1
216			min	-38.452	3	0	1	-15.4	4	0	1	-.016	4	0	1
217		14	max	713.272	1	0	1	.12	9	0	1	0	9	0	1
218			min	-38.403	3	0	1	-15.456	4	0	1	-.018	4	0	1
219		15	max	713.336	1	0	1	.12	9	0	1	0	9	0	1
220			min	-38.355	3	0	1	-15.512	4	0	1	-.019	4	0	1
221		16	max	713.401	1	0	1	.12	9	0	1	0	9	0	1
222			min	-38.306	3	0	1	-15.568	4	0	1	-.02	4	0	1
223		17	max	713.466	1	0	1	.12	9	0	1	0	9	0	1
224			min	-38.257	3	0	1	-15.624	4	0	1	-.022	4	0	1
225		18	max	713.53	1	0	1	.12	9	0	1	0	9	0	1
226			min	-38.209	3	0	1	-15.68	4	0	1	-.023	4	0	1
227		19	max	713.595	1	0	1	.12	9	0	1	0	9	0	1
228			min	-38.16	3	0	1	-15.736	4	0	1	-.025	4	0	1
229	M10	1	max	195.535	2	.687	4	1.11	5	0	1	0	1	0	1
230			min	-273.066	3	.175	15	-.102	1	-.001	5	0	3	0	1
231		2	max	195.661	2	.635	4	.995	5	0	1	0	4	0	15
232			min	-272.971	3	.163	15	-.102	1	-.001	5	0	3	0	4
233		3	max	195.787	2	.584	4	.881	5	0	1	0	4	0	15
234			min	-272.877	3	.151	15	-.102	1	-.001	5	0	3	0	4
235		4	max	195.913	2	.533	4	.767	5	0	1	0	4	0	15
236			min	-272.782	3	.139	15	-.102	1	-.001	5	0	3	0	4
237		5	max	196.039	2	.482	4	.652	5	0	1	0	4	0	15
238			min	-272.688	3	.127	15	-.102	1	-.001	5	0	3	0	4
239		6	max	196.165	2	.431	4	.538	5	0	1	0	4	0	15
240			min	-272.594	3	.115	15	-.102	1	-.001	5	0	3	0	4
241		7	max	196.291	2	.38	4	.423	5	0	1	0	4	0	15
242			min	-272.499	3	.103	15	-.102	1	-.001	5	0	3	0	4
243		8	max	196.417	2	.329	4	.309	5	0	1	0	4	0	15
244			min	-272.405	3	.091	15	-.102	1	-.001	5	0	3	0	4
245		9	max	196.542	2	.277	4	.194	5	0	1	0	5	0	15
246			min	-272.31	3	.079	15	-.102	1	-.001	5	0	3	0	4
247		10	max	196.668	2	.226	4	.08	5	0	1	0	5	0	15
248			min	-272.216	3	.06	12	-.102	1	-.001	5	0	3	0	4
249		11	max	196.794	2	.175	4	.002	10	0	1	0	5	0	15
250			min	-272.122	3	.04	12	-.102	1	-.001	5	0	3	0	4
251		12	max	196.92	2	.124	4	.002	10	0	1	0	5	0	15
252			min	-272.027	3	.02	12	-.163	4	-.001	5	0	3	0	4
253		13	max	197.046	2	.073	4	.002	10	0	1	0	5	0	15
254			min	-271.933	3	-.001	3	-.277	4	-.001	5	0	3	0	4
255		14	max	197.172	2	.028	5	.002	10	0	1	0	5	0	15
256			min	-271.838	3	-.031	3	-.392	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	197.298	2	.009	5	.002	10	0	1	0	5	0	15
258		min	-271.744	3	-.061	3	-.506	4	-.001	5	0	3	0	4
259	16	max	197.423	2	-.005	15	.002	10	0	1	0	5	0	15
260		min	-271.65	3	-.091	3	-.62	4	-.001	5	0	3	0	4
261	17	max	197.549	2	-.017	15	.002	10	0	1	0	5	0	15
262		min	-271.555	3	-.133	6	-.735	4	-.001	5	0	3	0	4
263	18	max	197.675	2	-.029	15	.002	10	0	1	0	5	0	12
264		min	-271.461	3	-.184	6	-.849	4	-.001	5	0	3	0	4
265	19	max	197.801	2	-.041	15	.002	10	0	1	0	5	0	12
266		min	-271.366	3	-.235	6	-.964	4	-.001	5	0	3	0	4
267	M11	1	max	183.198	2	1.746	.125	1	0	4	0	5	0	2
268		min	-174.315	3	.405	15	-1.267	5	0	10	0	1	0	15
269	2	max	183.129	2	1.57	6	.125	1	0	4	0	3	0	2
270		min	-174.367	3	.363	15	-1.133	5	0	10	0	1	0	12
271	3	max	183.059	2	1.393	6	.125	1	0	4	0	3	0	2
272		min	-174.419	3	.322	15	-.999	5	0	10	0	1	0	3
273	4	max	182.99	2	1.216	6	.125	1	0	4	0	3	0	15
274		min	-174.471	3	.28	15	-.866	5	0	10	0	1	0	4
275	5	max	182.921	2	1.039	6	.125	1	0	4	0	3	0	15
276		min	-174.523	3	.239	15	-.732	5	0	10	0	1	0	4
277	6	max	182.852	2	.862	6	.125	1	0	4	0	3	0	15
278		min	-174.575	3	.197	15	-.598	5	0	10	0	4	0	4
279	7	max	182.782	2	.685	6	.125	1	0	4	0	3	0	15
280		min	-174.627	3	.155	15	-.465	5	0	10	0	4	0	4
281	8	max	182.713	2	.509	6	.125	1	0	4	0	3	0	15
282		min	-174.679	3	.114	15	-.331	5	0	10	0	4	-.001	4
283	9	max	182.644	2	.332	6	.125	1	0	4	0	3	0	15
284		min	-174.731	3	.072	15	-.197	5	0	10	0	4	-.001	4
285	10	max	182.574	2	.156	2	.125	1	0	4	0	3	0	15
286		min	-174.783	3	.031	15	-.064	5	0	10	0	4	-.001	4
287	11	max	182.505	2	.018	2	.125	1	0	4	0	3	0	15
288		min	-174.835	3	-.041	3	-.056	3	0	10	0	4	-.001	4
289	12	max	182.436	2	-.052	15	.235	4	0	4	0	3	0	15
290		min	-174.887	3	-.199	4	-.056	3	0	10	0	4	-.001	4
291	13	max	182.366	2	-.094	15	.369	4	0	4	0	3	0	15
292		min	-174.938	3	-.376	4	-.056	3	0	10	0	4	-.001	4
293	14	max	182.297	2	-.136	15	.502	4	0	4	0	3	0	15
294		min	-174.99	3	-.553	4	-.056	3	0	10	0	4	-.001	4
295	15	max	182.228	2	-.177	15	.636	4	0	4	0	3	0	15
296		min	-175.042	3	-.73	4	-.056	3	0	10	0	4	0	4
297	16	max	182.158	2	-.219	15	.77	4	0	4	0	3	0	15
298		min	-175.094	3	-.907	4	-.056	3	0	10	0	5	0	4
299	17	max	182.089	2	-.26	15	.904	4	0	4	0	3	0	15
300		min	-175.146	3	-1.083	4	-.056	3	0	10	0	5	0	4
301	18	max	182.02	2	-.302	15	1.037	4	0	4	0	3	0	15
302		min	-175.198	3	-1.26	4	-.056	3	0	10	0	10	0	4
303	19	max	181.95	2	-.343	15	1.171	4	0	4	0	4	0	1
304		min	-175.25	3	-1.437	4	-.056	3	0	10	0	10	0	1
305	M12	1	max	255.193	1	0	.604	1	0	1	0	4	0	1
306		min	-.017	15	0	1	-13.552	5	0	1	0	3	0	1
307	2	max	255.257	1	0	1	.604	1	0	1	0	1	0	1
308		min	.002	15	0	1	-13.608	5	0	1	-.001	5	0	1
309	3	max	255.322	1	0	1	.604	1	0	1	0	1	0	1
310		min	.022	15	0	1	-13.664	5	0	1	-.002	5	0	1
311	4	max	255.387	1	0	1	.604	1	0	1	0	1	0	1
312		min	.041	15	0	1	-13.72	5	0	1	-.004	5	0	1
313	5	max	255.452	1	0	1	.604	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	.061	15	0	1	-13.776	5	0	1	-.005	5	0	1
315		6	max	255.516	1	0	1	.604	1	0	1	0	1	0	1
316			min	.08	15	0	1	-13.832	5	0	1	-.006	5	0	1
317		7	max	255.581	1	0	1	.604	1	0	1	0	1	0	1
318			min	.1	15	0	1	-13.889	5	0	1	-.007	5	0	1
319		8	max	255.646	1	0	1	.604	1	0	1	0	1	0	1
320			min	.119	15	0	1	-13.945	5	0	1	-.009	5	0	1
321		9	max	255.71	1	0	1	.604	1	0	1	0	1	0	1
322			min	.139	15	0	1	-14.001	5	0	1	-.01	5	0	1
323		10	max	255.775	1	0	1	.604	1	0	1	0	1	0	1
324			min	.159	15	0	1	-14.057	5	0	1	-.011	5	0	1
325		11	max	255.84	1	0	1	.604	1	0	1	0	1	0	1
326			min	.178	15	0	1	-14.113	5	0	1	-.012	5	0	1
327		12	max	255.905	1	0	1	.604	1	0	1	0	1	0	1
328			min	.198	15	0	1	-14.169	5	0	1	-.014	5	0	1
329		13	max	255.969	1	0	1	.604	1	0	1	0	1	0	1
330			min	.217	15	0	1	-14.225	5	0	1	-.015	5	0	1
331		14	max	256.034	1	0	1	.604	1	0	1	0	1	0	1
332			min	.237	15	0	1	-14.281	5	0	1	-.016	5	0	1
333		15	max	256.099	1	0	1	.604	1	0	1	0	1	0	1
334			min	.256	15	0	1	-14.337	5	0	1	-.017	5	0	1
335		16	max	256.163	1	0	1	.604	1	0	1	0	1	0	1
336			min	.276	15	0	1	-14.393	5	0	1	-.019	5	0	1
337		17	max	256.228	1	0	1	.604	1	0	1	0	1	0	1
338			min	.295	15	0	1	-14.449	5	0	1	-.02	5	0	1
339		18	max	256.293	1	0	1	.604	1	0	1	0	1	0	1
340			min	.315	15	0	1	-14.505	5	0	1	-.021	5	0	1
341		19	max	256.357	1	0	1	.604	1	0	1	0	1	0	1
342			min	.334	15	0	1	-14.561	5	0	1	-.023	5	0	1
343	M1	1	max	70.534	1	330.638	3	.814	10	0	2	.03	1	0	2
344			min	4.957	10	-213.689	2	-16.088	4	0	3	-.002	10	0	3
345		2	max	70.673	1	330.457	3	.814	10	0	2	.027	1	.047	2
346			min	5.074	10	-213.93	2	-15.846	4	0	3	-.001	10	-.072	3
347		3	max	87.907	3	4.592	4	.812	10	0	10	.023	1	.092	2
348			min	-17.183	10	-24.227	2	-15.101	1	0	1	-.001	10	-.142	3
349		4	max	88.012	3	4.281	4	.812	10	0	10	.02	1	.098	2
350			min	-17.066	10	-24.468	2	-15.101	1	0	1	-.001	10	-.14	3
351		5	max	88.117	3	3.975	14	.812	10	0	10	.016	1	.103	2
352			min	-16.95	10	-24.71	2	-15.101	1	0	1	0	10	-.137	3
353		6	max	88.222	3	3.737	14	.812	10	0	10	.013	1	.108	2
354			min	-16.834	10	-24.952	2	-15.101	1	0	1	0	10	-.134	3
355		7	max	88.326	3	3.5	14	.812	10	0	10	.01	1	.114	2
356			min	-16.717	10	-25.194	2	-15.101	1	0	1	0	10	-.132	3
357		8	max	88.431	3	3.262	14	.812	10	0	10	.007	1	.119	2
358			min	-16.601	10	-25.436	2	-15.101	1	0	1	0	10	-.129	3
359		9	max	88.536	3	3.024	14	.812	10	0	10	.003	1	.125	2
360			min	-16.485	10	-25.678	2	-15.101	1	0	1	0	10	-.126	3
361		10	max	88.64	3	2.787	14	.812	10	0	10	.002	3	.13	2
362			min	-16.368	10	-25.919	2	-15.101	1	0	1	0	10	-.123	3
363		11	max	88.745	3	2.549	14	.812	10	0	10	0	3	.136	2
364			min	-16.252	10	-26.161	2	-15.101	1	0	1	-.003	1	-.12	3
365		12	max	88.85	3	2.312	14	.812	10	0	10	0	10	.142	2
366			min	-16.136	10	-26.403	2	-15.101	1	0	1	-.006	1	-.117	3
367		13	max	88.955	3	2.074	14	.812	10	0	10	0	10	.147	2
368			min	-16.019	10	-26.645	2	-15.101	1	0	1	-.01	1	-.114	3
369		14	max	89.059	3	1.836	14	.812	10	0	10	0	10	.153	2
370			min	-15.903	10	-26.887	2	-15.101	1	0	1	-.013	1	-.111	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	89.164	3	1.613	9	.812	10	0	10	0	10	.159	2
372		min	-15.786	10	-27.129	2	-15.101	1	0	1	-.016	1	-.108	3
373	16	max	86.188	2	124.768	2	.817	10	0	1	.001	10	.164	2
374		min	-5.761	3	-160.656	3	-15.209	1	0	5	-.02	1	-.104	3
375	17	max	86.327	2	124.526	2	.817	10	0	1	.001	10	.137	2
376		min	-5.656	3	-160.838	3	-15.209	1	0	5	-.023	1	-.069	3
377	18	max	-4.753	12	316.063	2	.854	10	0	5	.001	10	.069	2
378		min	-70.672	1	-157.968	3	-24.105	4	0	2	-.026	1	-.035	3
379	19	max	-4.683	12	315.821	2	.854	10	0	5	.002	10	0	2
380		min	-70.532	1	-158.149	3	-23.863	4	0	2	-.03	1	0	3
381	M5	1	max	177.447	1	1046.757	3	0	11	0	.028	4	0	3
382		min	-4.325	3	-666.658	2	-79.58	3	0	3	0	11	0	2
383	2	max	177.586	1	1046.575	3	0	11	0	9	.024	4	.144	2
384		min	-4.22	3	-666.9	2	-79.58	3	0	3	-.005	3	-.226	3
385	3	max	237.124	3	4.737	9	8.536	3	0	3	.02	4	.286	2
386		min	-48.563	2	-80.008	2	-15.656	4	0	4	-.022	3	-.448	3
387	4	max	237.229	3	4.536	9	8.536	3	0	3	.017	4	.304	2
388		min	-48.424	2	-80.25	2	-15.414	4	0	4	-.02	3	-.438	3
389	5	max	237.334	3	4.334	9	8.536	3	0	3	.013	4	.321	2
390		min	-48.284	2	-80.492	2	-15.172	4	0	4	-.018	3	-.428	3
391	6	max	237.438	3	4.132	9	8.536	3	0	3	.01	4	.339	2
392		min	-48.144	2	-80.734	2	-14.93	4	0	4	-.016	3	-.417	3
393	7	max	237.543	3	3.931	9	8.536	3	0	3	.007	4	.356	2
394		min	-48.005	2	-80.976	2	-14.688	4	0	4	-.014	3	-.407	3
395	8	max	237.648	3	3.729	9	8.536	3	0	3	.004	4	.374	2
396		min	-47.865	2	-81.217	2	-14.446	4	0	4	-.013	3	-.396	3
397	9	max	237.753	3	3.528	9	8.536	3	0	3	0	4	.391	2
398		min	-47.726	2	-81.459	2	-14.204	4	0	4	-.011	3	-.385	3
399	10	max	237.857	3	3.326	9	8.536	3	0	3	0	1	.409	2
400		min	-47.586	2	-81.701	2	-13.962	4	0	4	-.009	3	-.375	3
401	11	max	237.962	3	3.125	9	8.536	3	0	3	0	2	.427	2
402		min	-47.446	2	-81.943	2	-13.72	4	0	4	-.007	3	-.364	3
403	12	max	238.067	3	2.923	9	8.536	3	0	3	0	2	.445	2
404		min	-47.307	2	-82.185	2	-13.478	4	0	4	-.008	4	-.353	3
405	13	max	238.171	3	2.722	9	8.536	3	0	3	0	2	.463	2
406		min	-47.167	2	-82.427	2	-13.236	4	0	4	-.011	4	-.343	3
407	14	max	238.276	3	2.52	9	8.536	3	0	3	0	2	.48	2
408		min	-47.027	2	-82.668	2	-12.994	4	0	4	-.014	4	-.332	3
409	15	max	238.381	3	2.319	9	8.536	3	0	3	0	3	.498	2
410		min	-46.888	2	-82.91	2	-12.752	4	0	4	-.017	4	-.321	3
411	16	max	266.777	2	395.749	2	8.512	3	0	3	.002	3	.512	2
412		min	-22.603	3	-449.066	3	-11.432	4	0	4	-.02	4	-.306	3
413	17	max	266.917	2	395.507	2	8.512	3	0	3	.004	3	.426	2
414		min	-22.498	3	-449.247	3	-11.19	4	0	4	-.022	4	-.209	3
415	18	max	-2.249	12	990.139	2	7.828	3	0	4	.005	3	.214	2
416		min	-177.598	1	-483.416	3	-25.406	5	0	9	-.028	4	-.104	3
417	19	max	-2.179	12	989.897	2	7.828	3	0	4	.007	3	0	3
418		min	-177.459	1	-483.597	3	-25.164	5	0	9	-.033	4	0	2
419	M9	1	max	70.509	1	330.545	3	108.045	4	0	.002	10	0	2
420		min	.959	15	-213.689	2	-.814	10	0	2	-.03	1	0	3
421	2	max	70.649	1	330.363	3	108.287	4	0	3	.022	5	.047	2
422		min	1.001	15	-213.93	2	-.814	10	0	2	-.026	1	-.072	3
423	3	max	87.301	3	4.023	9	15.032	1	0	1	.043	5	.092	2
424		min	-16.841	10	-24.201	2	-20.019	5	0	5	-.023	1	-.142	3
425	4	max	87.406	3	3.822	9	15.032	1	0	1	.039	5	.098	2
426		min	-16.725	10	-24.443	2	-19.777	5	0	5	-.02	1	-.139	3
427	5	max	87.51	3	3.62	9	15.032	1	0	1	.035	5	.103	2









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-117.344	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.349	3	.145	3	0	1	0	1	0	1
544			min	-117.415	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.262	3	.145	3	0	1	0	3	0	1
546			min	-117.485	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.175	3	.145	3	0	1	0	3	0	1
548			min	-117.556	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.087	3	.145	3	0	1	0	3	0	1
550			min	-117.626	3	0	1	0	1	0	3	0	1	0	3
551		10	max	0	1	0	1	.145	3	0	1	0	3	0	1
552			min	-117.697	3	0	1	0	1	0	3	0	1	0	3
553		11	max	0	1	0	1	.145	3	0	1	0	3	0	1
554			min	-117.767	3	-.087	3	0	1	0	3	0	1	0	3
555		12	max	0	1	0	1	.145	3	0	1	0	3	0	1
556			min	-117.838	3	-.175	3	0	1	0	3	0	1	0	3
557		13	max	0	1	0	1	.145	3	0	1	0	3	0	1
558			min	-117.908	3	-.262	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.145	3	0	1	0	3	0	1
560			min	-117.979	3	-.349	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.145	3	0	1	0	3	0	1
562			min	-118.049	3	-.436	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.145	3	0	1	0	3	0	1
564			min	-118.12	3	-.524	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.145	3	0	1	0	3	0	1
566			min	-118.19	3	-.611	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.145	3	0	1	0	3	0	1
568			min	-118.261	3	-.698	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.145	3	0	1	0	3	0	1
570			min	-118.331	3	-.785	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.053	4	.328	4	0	3	0	3	0	1
572			min	-169.312	4	0	2	-.06	3	0	1	0	4	0	1
573		2	max	0	2	1.825	4	.295	4	0	3	0	3	0	2
574			min	-169.288	4	0	2	-.06	3	0	1	0	4	0	4
575		3	max	0	2	1.597	4	.262	4	0	3	0	3	0	2
576			min	-169.265	4	0	2	-.06	3	0	1	0	4	-.001	4
577		4	max	0	2	1.369	4	.228	4	0	3	0	3	0	2
578			min	-169.242	4	0	2	-.06	3	0	1	0	1	-.001	4
579		5	max	0	2	1.14	4	.195	4	0	3	0	3	0	2
580			min	-169.219	4	0	2	-.06	3	0	1	0	1	-.002	4
581		6	max	0	2	.912	4	.162	4	0	3	0	3	0	2
582			min	-169.196	4	0	2	-.06	3	0	1	0	1	-.002	4
583		7	max	0	2	.684	4	.129	4	0	3	0	3	0	2
584			min	-169.173	4	0	2	-.06	3	0	1	0	1	-.002	4
585		8	max	0	2	.456	4	.096	4	0	3	0	5	0	2
586			min	-169.15	4	0	2	-.06	3	0	1	0	1	-.002	4
587		9	max	0	2	.228	4	.063	4	0	3	0	5	0	2
588			min	-169.127	4	0	2	-.06	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.04	1	0	3	0	5	0	2
590			min	-169.103	4	0	1	-.06	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.04	1	0	3	0	5	0	2
592			min	-169.08	4	-.228	4	-.06	3	0	1	0	1	-.003	4
593		12	max	.068	1	0	2	.04	1	0	3	0	5	0	2
594			min	-169.057	4	-.456	4	-.06	3	0	1	0	1	-.002	4
595		13	max	.162	1	0	2	.04	1	0	3	0	5	0	2
596			min	-169.034	4	-.684	4	-.073	5	0	1	0	3	-.002	4
597		14	max	.256	1	0	2	.04	1	0	3	0	5	0	2
598			min	-169.011	4	-.912	4	-.106	5	0	1	0	3	-.002	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.35	1	0	2	.04	1	0	3	0	5	0	2
600		min	-168.988	4	-1.14	4	-.139	5	0	1	0	3	-.002	4
601	16	max	.444	1	0	2	.04	1	0	3	0	1	0	2
602		min	-168.965	4	-1.369	4	-.172	5	0	1	0	3	-.001	4
603	17	max	.538	1	0	2	.04	1	0	3	0	1	0	2
604		min	-169.017	5	-1.597	4	-.205	5	0	1	0	3	-.001	4
605	18	max	.632	1	0	2	.04	1	0	3	0	1	0	2
606		min	-169.071	5	-1.825	4	-.238	5	0	1	0	5	0	4
607	19	max	.726	1	0	2	.04	1	0	3	0	1	0	1
608		min	-169.124	5	-2.053	4	-.271	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.009	2	.002	9	8.279e-4	5	NC	3	NC	1	
2			min	-.003	3	-.009	3	-.009	5	-2.438e-4	1	4314.383	2	NC	1	
3			2	max	.002	2	.008	2	.002	9	8.487e-4	5	NC	3	NC	1
4				min	-.003	3	-.009	3	-.009	5	-2.325e-4	1	4712.473	2	NC	1
5			3	max	.002	2	.008	2	.002	9	8.694e-4	5	NC	1	NC	1
6				min	-.003	3	-.008	3	-.009	5	-2.211e-4	1	5186.689	2	NC	1
7			4	max	.002	2	.007	2	.002	1	8.901e-4	5	NC	1	NC	1
8				min	-.003	3	-.008	3	-.009	5	-2.098e-4	1	5755.592	2	NC	1
9			5	max	.001	2	.006	2	.001	1	9.108e-4	5	NC	1	NC	1
10				min	-.003	3	-.008	3	-.008	5	-1.984e-4	1	6443.985	2	NC	1
11			6	max	.001	2	.005	2	.001	1	9.316e-4	5	NC	1	NC	1
12				min	-.002	3	-.007	3	-.008	5	-1.871e-4	1	7285.556	2	NC	1
13			7	max	.001	2	.005	2	.001	1	9.523e-4	5	NC	1	NC	1
14				min	-.002	3	-.007	3	-.008	5	-1.758e-4	1	8326.95	2	NC	1
15			8	max	.001	2	.004	2	.001	1	9.73e-4	5	NC	1	NC	1
16				min	-.002	3	-.006	3	-.007	5	-1.644e-4	1	9634.213	2	NC	1
17			9	max	.001	2	.003	2	0	1	9.938e-4	5	NC	1	NC	1
18				min	-.002	3	-.006	3	-.007	5	-1.531e-4	1	NC	1	NC	1
19			10	max	0	2	.003	2	0	1	1.014e-3	5	NC	1	NC	1
20				min	-.002	3	-.006	3	-.006	5	-1.417e-4	1	NC	1	NC	1
21		11	max	0	2	.002	2	0	1	1.035e-3	5	NC	1	NC	1	
22			min	-.002	3	-.005	3	-.006	5	-1.304e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	0	1	1.056e-3	5	NC	1	NC	1	
24			min	-.001	3	-.004	3	-.005	5	-1.191e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	1	1.077e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.005	5	-1.077e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	1	1.097e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.004	5	-9.638e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	1	1.118e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.003	5	-8.504e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	1	1.139e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-7.37e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	1	1.16e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-6.236e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	1	1.18e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-5.102e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.201e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.968e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.88e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.659e-4	5	NC	1	NC	1	
41		2	max	0	3	0	2	.003	5	2.603e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	9	-5.688e-4	5	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	3.326e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.717e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.009	5	4.049e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	9	-5.746e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.012	4	4.771e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.774e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.014	4	5.494e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.803e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.017	4	6.217e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-5.832e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.02	4	6.94e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-5.86e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.023	4	7.662e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-5.889e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.025	4	8.385e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-5.918e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.028	4	9.108e-5	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-5.946e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.03	4	9.831e-5	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	10	-5.975e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.032	4	1.055e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-6.004e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.034	4	1.128e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-6.032e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.037	4	1.2e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-6.061e-4	5	9487.261	2	NC	1
69		16	max	.002	3	.006	2	.039	4	1.272e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-6.09e-4	5	7995.276	2	NC	1
71		17	max	.002	3	.007	2	.041	4	1.344e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-6.118e-4	5	6850.444	2	NC	1
73		18	max	.002	3	.008	2	.043	4	1.417e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-6.147e-4	5	5960.796	2	NC	1
75		19	max	.002	3	.009	2	.044	4	1.489e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-6.176e-4	5	5262.552	2	NC	1
77	M4	1	max	.001	1	.01	2	0	10	3.027e-3	5	NC	1	NC	1
78			min	0	12	-.009	3	-.047	4	-1.788e-4	1	NC	1	412.798	4
79		2	max	.001	1	.01	2	0	10	3.027e-3	5	NC	1	NC	1
80			min	0	12	-.009	3	-.043	4	-1.788e-4	1	NC	1	449.945	4
81		3	max	.001	1	.009	2	0	10	3.027e-3	5	NC	1	NC	1
82			min	0	12	-.008	3	-.039	4	-1.788e-4	1	NC	1	494.15	4
83		4	max	.001	1	.009	2	0	10	3.027e-3	5	NC	1	NC	1
84			min	0	12	-.008	3	-.035	4	-1.788e-4	1	NC	1	547.272	4
85		5	max	0	1	.008	2	0	10	3.027e-3	5	NC	1	NC	1
86			min	0	12	-.007	3	-.032	4	-1.788e-4	1	NC	1	611.842	4
87		6	max	0	1	.008	2	0	10	3.027e-3	5	NC	1	NC	1
88			min	0	12	-.007	3	-.028	4	-1.788e-4	1	NC	1	691.382	4
89		7	max	0	1	.007	2	0	10	3.027e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.024	4	-1.788e-4	1	NC	1	790.906	4
91		8	max	0	1	.006	2	0	10	3.027e-3	5	NC	1	NC	1
92			min	0	12	-.006	3	-.021	4	-1.788e-4	1	NC	1	917.754	4
93		9	max	0	1	.006	2	0	10	3.027e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.018	4	-1.788e-4	1	NC	1	1083.03	4
95		10	max	0	1	.005	2	0	10	3.027e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.015	4	-1.788e-4	1	NC	1	1304.187	4
97		11	max	0	1	.005	2	0	10	3.027e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.012	4	-1.788e-4	1	NC	1	1609.965	4
99		12	max	0	1	.004	2	0	10	3.027e-3	5	NC	1	NC	1



RISA-3D Version 13.0.0      \.....\PVMini 60 Cell 1V 30° 120mph 30psf 3.75ft 7-05Pad      Page 34



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	8.55e-6	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-5.787e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.009	4	8.137e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-5.719e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.012	4	7.723e-6	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-5.652e-4	4	9601.845	2	NC	1
163		6	max	.002	3	.006	2	.015	4	2.336e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	-5.585e-4	4	7688.377	2	NC	1
165		7	max	.002	3	.007	2	.018	4	4.832e-5	3	NC	1	NC	1
166			min	-.002	2	-.011	3	0	9	-5.517e-4	4	6376.906	2	NC	1
167		8	max	.002	3	.009	2	.021	4	7.329e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	-5.45e-4	4	5413.612	2	NC	1
169		9	max	.002	3	.01	2	.023	4	9.825e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	9	-5.383e-4	4	4672.079	2	NC	1
171		10	max	.003	3	.011	2	.026	4	1.232e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	9	-5.315e-4	4	4082.222	2	NC	1
173		11	max	.003	3	.013	2	.029	4	1.482e-4	3	NC	3	NC	1
174			min	-.004	2	-.016	3	0	9	-5.248e-4	4	3601.9	2	NC	1
175		12	max	.003	3	.014	2	.031	4	1.731e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	9	-5.181e-4	4	3204.072	2	NC	1
177		13	max	.004	3	.016	2	.033	4	1.981e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	0	9	-5.113e-4	4	2870.429	2	NC	1
179		14	max	.004	3	.018	2	.036	4	2.231e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-5.046e-4	4	2588.003	2	NC	1
181		15	max	.004	3	.02	2	.038	4	2.48e-4	3	NC	3	NC	1
182			min	-.005	2	-.021	3	0	9	-4.979e-4	4	2347.258	2	NC	1
183		16	max	.005	3	.022	2	.04	4	2.73e-4	3	NC	3	NC	1
184			min	-.005	2	-.022	3	0	9	-4.911e-4	4	2140.97	2	NC	1
185		17	max	.005	3	.023	2	.042	4	2.98e-4	3	NC	3	NC	1
186			min	-.006	2	-.022	3	0	9	-4.844e-4	4	1963.531	2	NC	1
187		18	max	.005	3	.025	2	.044	4	3.229e-4	3	NC	3	NC	1
188			min	-.006	2	-.023	3	0	9	-4.777e-4	4	1810.507	2	NC	1
189		19	max	.005	3	.027	2	.046	4	3.479e-4	3	NC	3	NC	1
190			min	-.006	2	-.024	3	0	9	-4.709e-4	4	1678.348	2	NC	1
191	M8	1	max	.003	1	.033	2	0	9	2.884e-3	4	NC	1	NC	1
192			min	0	3	-.027	3	-.048	4	-2.545e-4	3	NC	1	405.519	4
193		2	max	.003	1	.031	2	0	9	2.884e-3	4	NC	1	NC	1
194			min	0	3	-.025	3	-.044	4	-2.545e-4	3	NC	1	442.013	4
195		3	max	.003	1	.029	2	0	9	2.884e-3	4	NC	1	NC	1
196			min	0	3	-.024	3	-.04	4	-2.545e-4	3	NC	1	485.441	4
197		4	max	.003	1	.027	2	0	9	2.884e-3	4	NC	1	NC	1
198			min	0	3	-.022	3	-.036	4	-2.545e-4	3	NC	1	537.629	4
199		5	max	.003	1	.025	2	0	9	2.884e-3	4	NC	1	NC	1
200			min	0	3	-.021	3	-.032	4	-2.545e-4	3	NC	1	601.065	4
201		6	max	.002	1	.024	2	0	9	2.884e-3	4	NC	1	NC	1
202			min	0	3	-.019	3	-.028	4	-2.545e-4	3	NC	1	679.207	4
203		7	max	.002	1	.022	2	0	9	2.884e-3	4	NC	1	NC	1
204			min	0	3	-.018	3	-.025	4	-2.545e-4	3	NC	1	776.983	4
205		8	max	.002	1	.02	2	0	9	2.884e-3	4	NC	1	NC	1
206			min	0	3	-.016	3	-.021	4	-2.545e-4	3	NC	1	901.605	4
207		9	max	.002	1	.018	2	0	9	2.884e-3	4	NC	1	NC	1
208			min	0	3	-.015	3	-.018	4	-2.545e-4	3	NC	1	1063.981	4
209		10	max	.002	1	.016	2	0	9	2.884e-3	4	NC	1	NC	1
210			min	0	3	-.013	3	-.015	4	-2.545e-4	3	NC	1	1281.257	4
211		11	max	.002	1	.014	2	0	9	2.884e-3	4	NC	1	NC	1
212			min	0	3	-.012	3	-.012	4	-2.545e-4	3	NC	1	1581.672	4
213		12	max	.001	1	.013	2	0	9	2.884e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.005	4	5.348e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-5.534e-4	4	NC	1	NC	1
273	4	max	0	3	0	2	.007	4	2.922e-5	3	NC	1	NC	1
274		min	0	2	-.003	3	-.001	3	-5.909e-4	4	NC	1	NC	1
275	5	max	0	3	0	2	.01	4	4.955e-6	3	NC	1	NC	1
276		min	0	2	-.003	3	-.002	3	-6.284e-4	4	NC	1	NC	1
277	6	max	0	3	0	2	.012	4	2.866e-6	10	NC	1	NC	1
278		min	0	2	-.004	3	-.002	3	-6.659e-4	4	NC	1	NC	1
279	7	max	0	3	0	2	.015	5	3.249e-6	10	NC	1	NC	1
280		min	0	2	-.005	3	-.002	3	-7.034e-4	4	NC	1	NC	1
281	8	max	0	3	0	2	.017	5	3.632e-6	10	NC	1	NC	1
282		min	0	2	-.006	3	-.002	3	-7.409e-4	4	NC	1	NC	1
283	9	max	0	3	.001	2	.02	5	4.015e-6	10	NC	1	NC	1
284		min	0	2	-.006	3	-.003	3	-7.784e-4	4	NC	1	NC	1
285	10	max	0	3	.002	2	.022	5	4.398e-6	10	NC	1	NC	1
286		min	-.001	2	-.007	3	-.003	3	-8.159e-4	4	NC	1	NC	1
287	11	max	.001	3	.002	2	.024	5	4.781e-6	10	NC	1	NC	1
288		min	-.001	2	-.007	3	-.003	3	-8.535e-4	4	NC	1	NC	1
289	12	max	.001	3	.003	2	.026	5	5.164e-6	10	NC	1	NC	1
290		min	-.001	2	-.007	3	-.003	3	-8.91e-4	4	NC	1	NC	1
291	13	max	.001	3	.003	2	.028	5	5.547e-6	10	NC	1	NC	1
292		min	-.001	2	-.008	3	-.003	3	-9.285e-4	4	NC	1	NC	1
293	14	max	.001	3	.004	2	.03	5	5.93e-6	10	NC	1	NC	1
294		min	-.001	2	-.008	3	-.003	3	-9.66e-4	4	NC	1	NC	1
295	15	max	.002	3	.005	2	.032	5	6.313e-6	10	NC	1	NC	1
296		min	-.002	2	-.008	3	-.003	3	-1.004e-3	4	9500.451	2	NC	1
297	16	max	.002	3	.006	2	.034	5	6.696e-6	10	NC	1	NC	1
298		min	-.002	2	-.008	3	-.003	3	-1.041e-3	4	8005.267	2	NC	1
299	17	max	.002	3	.007	2	.036	5	7.079e-6	10	NC	1	NC	1
300		min	-.002	2	-.008	3	-.003	3	-1.079e-3	4	6858.235	2	NC	1
301	18	max	.002	3	.008	2	.038	5	7.462e-6	10	NC	1	NC	1
302		min	-.002	2	-.008	3	-.003	3	-1.116e-3	4	5967.039	2	NC	1
303	19	max	.002	3	.009	2	.04	5	7.845e-6	10	NC	3	NC	1
304		min	-.002	2	-.008	3	-.002	3	-1.154e-3	4	5267.687	2	NC	1
305	M12	1	max	.001	1	.01	.002	1	3.493e-3	4	NC	1	NC	2
306		min	0	15	-.009	3	-.044	5	-9.493e-6	10	NC	1	439.803	5
307	2	max	.001	1	.01	2	.002	1	3.493e-3	4	NC	1	NC	1
308		min	0	15	-.009	3	-.04	5	-9.493e-6	10	NC	1	479.369	5
309	3	max	.001	1	.009	2	.002	1	3.493e-3	4	NC	1	NC	1
310		min	0	15	-.008	3	-.037	5	-9.493e-6	10	NC	1	526.451	5
311	4	max	.001	1	.009	2	.001	1	3.493e-3	4	NC	1	NC	1
312		min	0	15	-.008	3	-.033	5	-9.493e-6	10	NC	1	583.03	5
313	5	max	0	1	.008	2	.001	1	3.493e-3	4	NC	1	NC	1
314		min	0	15	-.007	3	-.03	5	-9.493e-6	10	NC	1	651.8	5
315	6	max	0	1	.008	2	.001	1	3.493e-3	4	NC	1	NC	1
316		min	0	15	-.007	3	-.026	5	-9.493e-6	10	NC	1	736.512	5
317	7	max	0	1	.007	2	.001	1	3.493e-3	4	NC	1	NC	1
318		min	0	15	-.006	3	-.023	5	-9.493e-6	10	NC	1	842.505	5
319	8	max	0	1	.006	2	0	1	3.493e-3	4	NC	1	NC	1
320		min	0	15	-.006	3	-.02	5	-9.493e-6	10	NC	1	977.597	5
321	9	max	0	1	.006	2	0	1	3.493e-3	4	NC	1	NC	1
322		min	0	15	-.005	3	-.017	5	-9.493e-6	10	NC	1	1153.61	5
323	10	max	0	1	.005	2	0	1	3.493e-3	4	NC	1	NC	1
324		min	0	15	-.005	3	-.014	5	-9.493e-6	10	NC	1	1389.129	5
325	11	max	0	1	.005	2	0	1	3.493e-3	4	NC	1	NC	1
326		min	0	15	-.004	3	-.011	5	-9.493e-6	10	NC	1	1714.76	5
327	12	max	0	1	.004	2	0	1	3.493e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.009	5	-9.493e-6	10	NC	1	2183.884	5
329		max	0	1	.003	2	0	1	3.493e-3	4	NC	1	NC	1
330		min	0	15	-.003	3	-.007	5	-9.493e-6	10	NC	1	2896.62	5
331		max	0	1	.003	2	0	1	3.493e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.005	5	-9.493e-6	10	NC	1	4059.473	5
333		max	0	1	.002	2	0	1	3.493e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.003	5	-9.493e-6	10	NC	1	6157.545	5
335		max	0	1	.002	2	0	1	3.493e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.002	5	-9.493e-6	10	NC	1	NC	1
337		max	0	1	.001	2	0	1	3.493e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	0	5	-9.493e-6	10	NC	1	NC	1
339		max	0	1	0	2	0	1	3.493e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	-9.493e-6	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.493e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	-9.493e-6	10	NC	1	NC	1
343	M1	max	.008	3	.024	3	.006	5	5.216e-3	2	NC	1	NC	1
344		min	-.009	2	-.02	2	0	9	-7.616e-3	3	NC	1	NC	1
345		max	.008	3	.014	3	.007	5	2.575e-3	2	NC	4	NC	1
346		min	-.009	2	-.012	2	-.002	9	-3.748e-3	3	4868.141	3	NC	1
347		max	.008	3	.005	3	.009	5	2.674e-4	5	NC	4	NC	1
348		min	-.009	2	-.004	2	-.002	9	-1.13e-4	1	2521.594	3	NC	1
349		max	.008	3	.003	2	.012	5	2.68e-4	5	NC	4	NC	1
350		min	-.009	2	-.003	3	-.002	1	-9.337e-5	1	1796.545	3	8114.694	5
351		max	.008	3	.009	2	.014	5	2.685e-4	5	NC	4	NC	1
352		min	-.009	2	-.009	3	-.003	1	-7.377e-5	1	1450.784	3	5748.716	5
353		max	.008	3	.015	2	.016	5	2.691e-4	5	NC	4	NC	1
354		min	-.009	2	-.014	3	-.002	1	-5.72e-5	9	1257.795	3	4382.04	5
355		max	.008	3	.019	2	.019	5	2.696e-4	5	NC	4	NC	1
356		min	-.008	2	-.018	3	-.002	1	-4.179e-5	9	1143.548	3	3505.531	5
357		max	.008	3	.022	2	.022	5	2.702e-4	5	NC	4	NC	1
358		min	-.008	2	-.021	3	-.002	9	-2.638e-5	9	1057.31	2	2903.384	5
359		max	.008	3	.024	2	.025	5	2.708e-4	5	NC	4	NC	1
360		min	-.008	2	-.022	3	-.001	9	-1.098e-5	9	1003.529	2	2469.11	5
361		max	.008	3	.025	2	.028	5	2.752e-4	4	NC	4	NC	1
362		min	-.008	2	-.023	3	0	9	-9.6e-7	10	977.808	2	2129.171	4
363		max	.008	3	.024	2	.031	4	2.81e-4	4	NC	4	NC	1
364		min	-.008	2	-.022	3	0	9	-2.054e-6	10	977.197	2	1868.675	4
365		max	.008	3	.023	2	.034	4	2.868e-4	4	NC	4	NC	1
366		min	-.008	2	-.02	3	0	10	-3.147e-6	10	1002.711	2	1667.216	4
367		max	.008	3	.02	2	.037	4	2.926e-4	4	NC	4	NC	1
368		min	-.008	2	-.017	3	0	10	-4.241e-6	10	1059.928	2	1508.827	4
369		max	.008	3	.016	2	.039	4	2.983e-4	4	NC	4	NC	1
370		min	-.008	2	-.014	3	0	10	-5.335e-6	10	1161.845	2	1382.826	4
371		max	.008	3	.01	2	.042	4	3.041e-4	4	NC	4	NC	1
372		min	-.008	2	-.009	3	0	10	-6.428e-6	10	1337.12	2	1281.859	4
373		max	.008	3	.003	2	.045	4	4.733e-4	4	NC	4	NC	1
374		min	-.008	2	-.003	3	0	10	-7.238e-6	10	1656.156	2	1200.745	4
375		max	.008	3	.004	3	.047	4	4.53e-3	4	NC	4	NC	1
376		min	-.008	2	-.006	2	0	10	-4.303e-6	9	2346.669	2	1135.861	4
377		max	.008	3	.012	3	.049	4	3.74e-3	2	NC	4	NC	1
378		min	-.008	2	-.016	2	0	10	-2.006e-3	3	4548.509	2	1084.34	4
379		max	.008	3	.02	3	.05	4	7.544e-3	2	NC	1	NC	1
380		min	-.008	2	-.027	2	0	9	-4.124e-3	3	NC	1	1045.434	4
381	M5	max	.024	3	.074	3	.006	5	1.833e-5	4	NC	1	NC	1
382		min	-.027	2	-.062	2	0	9	0	1	NC	1	NC	1
383		max	.024	3	.044	3	.007	5	1.369e-4	3	NC	4	NC	1
384		min	-.027	2	-.036	2	0	9	-1.182e-5	9	1576.198	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.024	3	.015	3	.009	5	2.615e-4	3	NC	5	NC	1
386		min	-.027	2	-.011	2	0	9	-2.352e-5	9	816.958	3	NC	1
387	4	max	.024	3	.01	2	.012	5	2.559e-4	5	NC	5	NC	1
388		min	-.027	2	-.009	3	0	9	-2.237e-5	9	582.924	3	NC	1
389	5	max	.024	3	.029	2	.014	5	2.65e-4	5	NC	5	NC	1
390		min	-.027	2	-.028	3	0	9	-2.122e-5	9	471.472	3	9416.324	3
391	6	max	.024	3	.046	2	.017	5	2.741e-4	5	NC	5	NC	1
392		min	-.027	2	-.044	3	0	9	-2.007e-5	9	409.402	3	8505.213	3
393	7	max	.024	3	.059	2	.02	5	2.833e-4	5	NC	5	NC	1
394		min	-.027	2	-.056	3	0	9	-1.891e-5	9	366.433	2	8090.186	3
395	8	max	.024	3	.068	2	.023	5	2.924e-4	5	NC	5	NC	1
396		min	-.027	2	-.064	3	0	9	-1.776e-5	9	337.663	2	8006.242	3
397	9	max	.024	3	.075	2	.026	5	3.015e-4	5	NC	5	NC	1
398		min	-.027	2	-.069	3	0	9	-1.661e-5	9	320.436	2	8191.875	3
399	10	max	.024	3	.077	2	.029	5	3.107e-4	5	NC	5	NC	1
400		min	-.027	2	-.07	3	0	9	-1.546e-5	9	312.189	2	8641.202	3
401	11	max	.023	3	.076	2	.032	4	3.198e-4	5	NC	5	NC	1
402		min	-.027	2	-.068	3	0	9	-1.431e-5	9	311.973	2	9392.391	3
403	12	max	.023	3	.071	2	.035	4	3.289e-4	5	NC	5	NC	1
404		min	-.027	2	-.062	3	0	9	-1.316e-5	9	320.113	2	NC	1
405	13	max	.023	3	.062	2	.038	4	3.381e-4	5	NC	5	NC	1
406		min	-.027	2	-.053	3	0	9	-1.201e-5	9	338.388	2	NC	1
407	14	max	.023	3	.049	2	.041	4	3.472e-4	5	NC	5	NC	1
408		min	-.027	2	-.041	3	0	9	-1.085e-5	9	370.952	2	NC	1
409	15	max	.023	3	.031	2	.043	4	3.566e-4	4	NC	5	NC	1
410		min	-.027	2	-.026	3	0	9	-9.704e-6	9	426.961	2	NC	1
411	16	max	.023	3	.009	2	.046	4	5.274e-4	4	NC	5	NC	1
412		min	-.027	2	-.008	3	0	9	-9.437e-6	9	528.917	2	NC	1
413	17	max	.023	3	.013	3	.048	4	4.531e-3	4	NC	5	NC	1
414		min	-.027	2	-.019	2	0	9	-3.023e-5	9	749.641	2	NC	1
415	18	max	.023	3	.036	3	.049	4	2.327e-3	4	NC	4	NC	1
416		min	-.027	2	-.05	2	0	9	-1.548e-5	9	1453.361	2	NC	1
417	19	max	.023	3	.06	3	.05	4	5.955e-6	5	NC	1	NC	1
418		min	-.027	2	-.083	2	0	9	-1.644e-6	3	NC	1	NC	1
419	M9	1	max	.008	.023	3	.005	5	7.635e-3	3	NC	1	NC	1
420		min	-.008	2	-.02	2	0	9	-5.216e-3	2	NC	1	NC	1
421	2	max	.008	3	.014	3	.005	4	3.754e-3	3	NC	4	NC	1
422		min	-.008	2	-.012	2	0	10	-2.575e-3	2	4870.486	3	NC	1
423	3	max	.008	3	.004	3	.005	4	8.846e-5	1	NC	4	NC	1
424		min	-.009	2	-.004	2	0	10	-5.465e-5	3	2522.828	3	NC	1
425	4	max	.008	3	.003	2	.006	4	7.011e-5	1	NC	4	NC	1
426		min	-.009	2	-.003	3	-.001	3	-5.984e-5	3	1797.396	3	NC	1
427	5	max	.008	3	.009	2	.007	4	5.176e-5	1	NC	4	NC	1
428		min	-.008	2	-.01	3	-.002	3	-6.503e-5	3	1451.415	3	9186.815	3
429	6	max	.008	3	.015	2	.008	4	3.341e-5	1	NC	4	NC	1
430		min	-.008	2	-.015	3	-.003	3	-7.022e-5	3	1258.282	3	7990.434	3
431	7	max	.008	3	.019	2	.01	4	1.507e-5	1	NC	4	NC	1
432		min	-.008	2	-.019	3	-.004	3	-7.541e-5	3	1143.93	3	7300.365	3
433	8	max	.008	3	.022	2	.013	4	4.06e-6	11	NC	4	NC	1
434		min	-.008	2	-.021	3	-.004	3	-8.06e-5	3	1057.592	2	5821.647	4
435	9	max	.008	3	.024	2	.016	4	0	10	NC	4	NC	1
436		min	-.008	2	-.023	3	-.005	3	-8.579e-5	3	1003.806	2	4324.998	4
437	10	max	.008	3	.025	2	.019	5	3.423e-6	5	NC	4	NC	1
438		min	-.008	2	-.023	3	-.005	3	-9.098e-5	3	978.087	2	3374.627	4
439	11	max	.008	3	.024	2	.022	5	9.96e-6	5	NC	4	NC	1
440		min	-.008	2	-.022	3	-.005	3	-9.617e-5	3	977.483	2	2731.787	4
441	12	max	.008	3	.023	2	.026	5	1.65e-5	5	NC	4	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.005	4	-.009	9	-.036	3	-4.108e-3	2	6181.837	2	1100.433	3
557	13	max	.001	3	.006	5	.027	2	4.92e-3	3	NC	4	5481.737	15
558		min	-.005	4	-.008	9	-.035	3	-4.477e-3	2	6694.605	2	1089.773	3
559	14	max	.001	3	.006	5	.024	2	5.297e-3	3	NC	3	7222.673	15
560		min	-.005	4	-.007	9	-.032	3	-4.847e-3	2	7549.005	2	1123.967	3
561	15	max	.001	3	.006	5	.019	1	5.674e-3	3	NC	3	NC	15
562		min	-.006	4	-.006	9	-.026	3	-5.217e-3	2	8969.765	2	1220.516	3
563	16	max	.002	3	.006	5	.014	1	6.051e-3	3	NC	1	NC	5
564		min	-.006	4	-.005	9	-.017	3	-5.586e-3	2	NC	1	1426.894	3
565	17	max	.002	3	.006	5	.005	1	6.428e-3	3	NC	1	NC	4
566		min	-.007	4	-.004	9	-.006	3	-5.956e-3	2	NC	1	1892.005	3
567	18	max	.002	3	.006	5	.01	3	6.805e-3	3	NC	1	NC	4
568		min	-.007	4	-.002	9	-.012	2	-6.326e-3	2	NC	1	3369.064	3
569	19	max	.002	3	.007	2	.029	3	7.182e-3	3	NC	1	NC	1
570		min	-.007	4	0	9	-.028	2	-6.696e-3	2	NC	1	NC	1
571	M16A	1	max	.001	2	.002	.009	3	2.042e-3	3	NC	1	NC	1
572		min	-.003	4	-.004	4	-.009	2	-2.114e-3	2	NC	1	NC	1
573	2	max	0	2	0	2	.002	3	1.967e-3	3	NC	1	NC	1
574		min	-.003	4	-.007	4	-.004	2	-2.017e-3	2	NC	1	9360.123	3
575	3	max	0	2	-.002	10	.003	1	1.892e-3	3	NC	1	NC	4
576		min	-.002	4	-.01	4	-.005	5	-1.921e-3	2	8548.484	4	5301.72	3
577	4	max	0	2	-.003	10	.005	1	1.816e-3	3	NC	1	NC	6
578		min	-.002	4	-.013	4	-.009	5	-1.824e-3	2	5864.759	4	4037.362	3
579	5	max	0	2	-.004	12	.007	1	1.741e-3	3	NC	3	NC	9
580		min	-.002	4	-.016	4	-.013	5	-1.728e-3	2	4576.331	4	3491.832	3
581	6	max	0	2	-.005	12	.008	1	1.666e-3	3	NC	3	NC	9
582		min	-.002	4	-.018	4	-.018	5	-1.631e-3	2	3851.466	4	3256.826	3
583	7	max	0	2	-.005	12	.009	1	1.591e-3	3	NC	12	NC	9
584		min	-.002	4	-.02	4	-.022	5	-1.535e-3	2	3415.556	4	2932.925	5
585	8	max	0	2	-.006	12	.009	1	1.516e-3	3	NC	12	NC	9
586		min	-.002	4	-.021	4	-.025	5	-1.439e-3	2	3153.944	4	2507.759	5
587	9	max	0	2	-.006	12	.009	1	1.441e-3	3	NC	12	NC	9
588		min	-.001	4	-.022	4	-.027	5	-1.342e-3	2	3013.129	4	2266.753	5
589	10	max	0	2	-.006	12	.008	1	1.366e-3	3	NC	12	NC	9
590		min	-.001	4	-.022	4	-.029	5	-1.246e-3	2	2968.582	4	2149.768	5
591	11	max	0	2	-.006	12	.007	1	1.291e-3	3	NC	12	NC	9
592		min	-.001	4	-.022	4	-.029	5	-1.149e-3	2	3013.129	4	2131.784	5
593	12	max	0	2	-.005	12	.005	1	1.216e-3	3	NC	12	NC	9
594		min	-.001	4	-.02	4	-.028	5	-1.053e-3	2	3153.944	4	2209.8	5
595	13	max	0	2	-.005	12	.004	1	1.141e-3	3	NC	12	NC	2
596		min	0	4	-.019	4	-.026	5	-9.564e-4	2	3415.556	4	2401.708	5
597	14	max	0	2	-.004	12	.003	1	1.065e-3	3	NC	3	NC	1
598		min	0	4	-.017	4	-.022	5	-8.6e-4	2	3851.466	4	2756.422	5
599	15	max	0	2	-.004	12	.002	1	9.903e-4	3	NC	3	NC	1
600		min	0	4	-.014	4	-.018	5	-7.635e-4	2	4576.331	4	3386.935	5
601	16	max	0	2	-.003	12	.001	9	9.152e-4	3	NC	1	NC	1
602		min	0	4	-.011	4	-.013	5	-6.671e-4	2	5864.759	4	4576.348	5
603	17	max	0	2	-.002	12	0	9	8.401e-4	3	NC	1	NC	1
604		min	0	4	-.007	4	-.009	5	-5.707e-4	2	8548.484	4	7213.843	5
605	18	max	0	2	0	12	0	3	8.908e-4	4	NC	1	NC	1
606		min	0	4	-.004	4	-.004	5	-4.742e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	9.517e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-3.778e-4	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.