

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	110 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 19.00 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.059 k-ft
P_n =	0.321 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	17%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.327 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.322 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	19%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.008 k-ft
P_n =	0.273 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	20%



A cross brace kit is required every 10 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

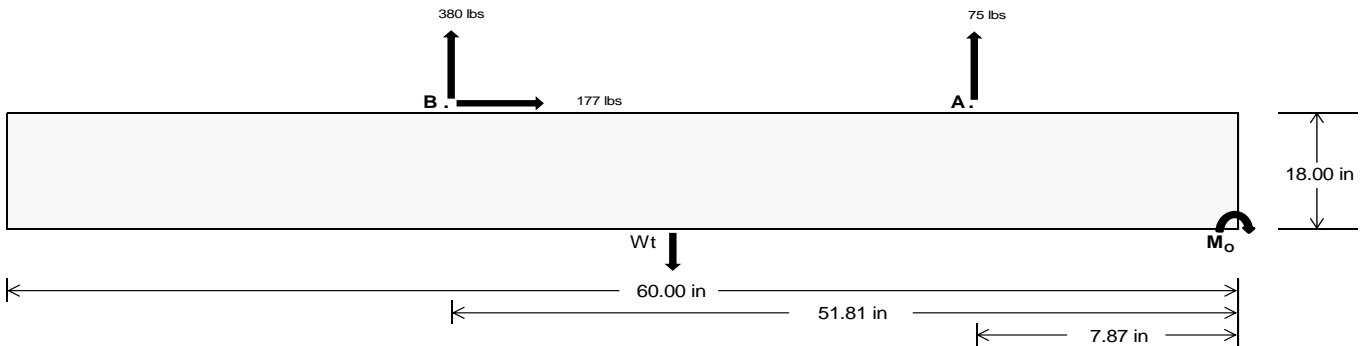
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	333.20	1653.89	k
Compressive Load =	2105.72	1632.85	k
Lateral Load =	47.72	768.26	k
Moment (Weak Axis) =	0.08	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23466.3$ in-lbs
Resisting Force Required = 782.21 lbs
S.F. = 1.67
Weight Required = 1303.69 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 177.14 lbs
Friction = 0.4
Weight Required = 442.85 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 177.14 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	810 lbs	810 lbs	810 lbs	810 lbs	567 lbs	567 lbs	567 lbs	567 lbs	973 lbs	973 lbs	973 lbs	973 lbs	-149 lbs	-149 lbs	-149 lbs	-149 lbs
F_B	595 lbs	595 lbs	595 lbs	595 lbs	499 lbs	499 lbs	499 lbs	499 lbs	775 lbs	775 lbs	775 lbs	775 lbs	-760 lbs	-760 lbs	-760 lbs	-760 lbs
F_V	74 lbs	74 lbs	74 lbs	74 lbs	321 lbs	321 lbs	321 lbs	321 lbs	291 lbs	291 lbs	291 lbs	291 lbs	-354 lbs	-354 lbs	-354 lbs	-354 lbs
P_{total}	3399 lbs	3490 lbs	3580 lbs	3671 lbs	3059 lbs	3150 lbs	3240 lbs	3331 lbs	3741 lbs	3832 lbs	3922 lbs	4013 lbs	287 lbs	341 lbs	396 lbs	450 lbs
M	524 lbs-ft	524 lbs-ft	524 lbs-ft	524 lbs-ft	619 lbs-ft	619 lbs-ft	619 lbs-ft	619 lbs-ft	822 lbs-ft	822 lbs-ft	822 lbs-ft	822 lbs-ft	575 lbs-ft	575 lbs-ft	575 lbs-ft	575 lbs-ft
e	0.15 ft	0.15 ft	0.15 ft	0.14 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.22 ft	0.21 ft	0.21 ft	0.21 ft	0.20 ft	2.00 ft	1.68 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	302.2 psf	298.5 psf	295.2 psf	292.1 psf	252.6 psf	251.1 psf	249.7 psf	248.4 psf	300.6 psf	296.9 psf	293.6 psf	290.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	439.4 psf	429.7 psf	420.9 psf	412.7 psf	414.8 psf	406.2 psf	398.3 psf	391.1 psf	515.7 psf	502.7 psf	490.8 psf	479.9 psf	210.2 psf	145.5 psf	126.0 psf	117.8 psf

Maximum Bearing Pressure = 516 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

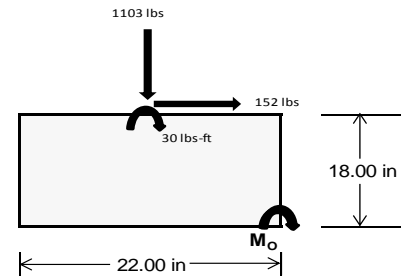
Overturning Check

$M_o = 753.0 \text{ ft-lbs}$
 Resisting Force Required = 821.44 lbs
 S.F. = 1.67
 Weight Required = 1369.07 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	145 lbs	207 lbs	93 lbs	414 lbs	1103 lbs	373 lbs	79 lbs	22 lbs	29 lbs
F_h	25 lbs	201 lbs	26 lbs	16 lbs	152 lbs	20 lbs	25 lbs	200 lbs	26 lbs
P_{total}	2614 lbs	2675 lbs	2561 lbs	2764 lbs	3453 lbs	2722 lbs	800 lbs	744 lbs	751 lbs
M	72 lbs-ft	341 lbs-ft	78 lbs-ft	48 lbs-ft	258 lbs-ft	61 lbs-ft	73 lbs-ft	340 lbs-ft	78 lbs-ft
e	0.03 ft	0.13 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.09 ft	0.46 ft	0.10 ft
$L/6$	0.31 ft	1.58 ft	1.77 ft	1.80 ft	1.68 ft	1.79 ft	1.65 ft	0.92 ft	1.63 ft
f_{min}	259.3 sqft	170.1 sqft	251.6 sqft	284.5 sqft	284.5 sqft	275.2 sqft	61.2 sqft	-40.4 sqft	54.2 sqft
f_{max}	311.0 psf	413.5 psf	307.1 psf	318.5 psf	468.8 psf	318.8 psf	113.4 psf	202.7 psf	109.7 psf



Maximum Bearing Pressure = 469 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.372 k
Allowable Uplift =	1.214 k
Utilization =	<u>31%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.106 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.620 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>28%</u>

Diagonal Strut

Maximum Axial Load =	0.327 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.322 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

Bracing

Maximum Axial Load =	0.273 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.125 in
	<u>0.125 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

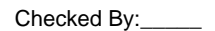
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 20° 110mph 30psf 8.5ft 7-10.rdb Page 20



RISA-3D Version 13.0.0 \...\...\PVMini 60 Cell 1V 20° 110mph 30psf 8.5ft 7-10.r Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	403.587	1	.071	2	.872	1	0	12	.002	1	0	15
30			min	-365.118	3	.014	15	-.358	5	-.001	1	0	3	0	6
31		16	max	403.693	1	.039	2	.872	1	0	12	.002	1	0	15
32			min	-365.038	3	-.006	9	-.454	5	-.001	1	0	3	0	6
33		17	max	403.8	1	.009	10	.872	1	0	12	.002	1	0	15
34			min	-364.958	3	-.034	1	-.551	5	-.001	1	0	3	0	6
35		18	max	403.906	1	-.015	15	.872	1	0	12	.002	1	0	15
36			min	-364.878	3	-.066	1	-.647	5	-.001	1	0	3	0	6
37		19	max	404.013	1	-.025	15	.872	1	0	12	.002	1	0	15
38			min	-364.798	3	-.104	4	-.744	5	-.001	1	0	3	0	6
39	M3	1	max	60.058	2	1.792	6	-.037	12	0	5	.003	1	0	6
40			min	-108.672	9	.421	15	-1.522	4	0	1	0	12	0	15
41		2	max	59.991	2	1.614	6	-.037	12	0	5	.002	1	0	6
42			min	-108.729	9	.379	15	-1.388	4	0	1	0	12	0	15
43		3	max	59.923	2	1.437	6	-.037	12	0	5	.002	1	0	2
44			min	-108.786	9	.337	15	-1.254	4	0	1	0	12	0	9
45		4	max	59.855	2	1.259	6	-.037	12	0	5	.002	1	0	15
46			min	-108.842	9	.296	15	-1.121	4	0	1	0	15	0	4
47		5	max	59.787	2	1.081	6	-.037	12	0	5	.002	1	0	15
48			min	-108.899	9	.254	15	-.987	4	0	1	0	5	0	4
49		6	max	59.719	2	.904	6	-.037	12	0	5	.002	1	0	15
50			min	-108.955	9	.212	15	-.853	4	0	1	0	5	0	4
51		7	max	59.651	2	.726	6	-.037	12	0	5	.001	1	0	15
52			min	-109.012	9	.17	15	-.847	1	0	1	0	5	0	4
53		8	max	59.583	2	.548	6	-.037	12	0	5	.001	1	0	15
54			min	-109.068	9	.128	15	-.847	1	0	1	0	5	-.001	4
55		9	max	59.516	2	.371	6	-.037	12	0	5	.001	1	0	15
56			min	-109.125	9	.087	15	-.847	1	0	1	0	5	-.001	4
57		10	max	59.448	2	.193	6	-.037	12	0	5	0	1	0	15
58			min	-109.181	9	.045	15	-.847	1	0	1	0	5	-.001	4
59		11	max	59.38	2	.029	2	-.01	15	0	5	0	1	0	15
60			min	-109.238	9	-.003	9	-.847	1	0	1	0	5	-.001	4
61		12	max	59.312	2	-.039	15	.116	5	0	5	0	1	0	15
62			min	-109.295	9	-.162	4	-.847	1	0	1	0	5	-.001	4
63		13	max	59.244	2	-.08	15	.25	5	0	5	0	1	0	15
64			min	-109.351	9	-.34	4	-.847	1	0	1	0	5	-.001	4
65		14	max	59.176	2	-.122	15	.384	5	0	5	0	1	0	15
66			min	-109.408	9	-.518	4	-.847	1	0	1	0	5	-.001	4
67		15	max	59.108	2	-.164	15	.517	5	0	5	0	1	0	15
68			min	-109.464	9	-.695	4	-.847	1	0	1	0	5	0	4
69		16	max	59.041	2	-.206	15	.651	5	0	5	0	12	0	15
70			min	-109.521	9	-.873	4	-.847	1	0	1	0	4	0	4
71		17	max	58.973	2	-.247	15	.784	5	0	5	0	12	0	15
72			min	-109.577	9	-1.051	4	-.847	1	0	1	0	1	0	4
73		18	max	58.905	2	-.289	15	.918	5	0	5	0	12	0	15
74			min	-109.634	9	-1.228	4	-.847	1	0	1	0	1	0	4
75		19	max	58.837	2	-.331	15	1.052	5	0	5	0	5	0	1
76			min	-109.69	9	-1.406	4	-.847	1	0	1	0	1	0	1
77	M4	1	max	566.859	1	0	1	-.146	12	0	1	0	5	0	1
78			min	-70.047	3	0	1	-36.169	4	0	1	0	1	0	1
79		2	max	566.923	1	0	1	-.146	12	0	1	0	12	0	1
80			min	-69.998	3	0	1	-36.225	4	0	1	-.003	4	0	1
81		3	max	566.988	1	0	1	-.146	12	0	1	0	12	0	1
82			min	-69.95	3	0	1	-36.281	4	0	1	-.006	4	0	1
83		4	max	567.053	1	0	1	-.146	12	0	1	0	12	0	1
84			min	-69.901	3	0	1	-36.338	4	0	1	-.01	4	0	1
85		5	max	567.118	1	0	1	-.146	12	0	1	0	12	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-69.852	3	0	1	-36.394	4	0	1	-.013	4	0	1
87	6	max	567.182	1	0	1	-.146	12	0	1	0	12	0	1
88		min	-69.804	3	0	1	-36.45	4	0	1	-.016	4	0	1
89	7	max	567.247	1	0	1	-.146	12	0	1	0	12	0	1
90		min	-69.755	3	0	1	-36.506	4	0	1	-.019	4	0	1
91	8	max	567.312	1	0	1	-.146	12	0	1	0	12	0	1
92		min	-69.707	3	0	1	-36.562	4	0	1	-.023	4	0	1
93	9	max	567.376	1	0	1	-.146	12	0	1	0	12	0	1
94		min	-69.658	3	0	1	-36.618	4	0	1	-.026	4	0	1
95	10	max	567.441	1	0	1	-.146	12	0	1	0	12	0	1
96		min	-69.61	3	0	1	-36.674	4	0	1	-.029	4	0	1
97	11	max	567.506	1	0	1	-.146	12	0	1	0	12	0	1
98		min	-69.561	3	0	1	-36.73	4	0	1	-.033	4	0	1
99	12	max	567.57	1	0	1	-.146	12	0	1	0	12	0	1
100		min	-69.513	3	0	1	-36.786	4	0	1	-.036	4	0	1
101	13	max	567.635	1	0	1	-.146	12	0	1	0	12	0	1
102		min	-69.464	3	0	1	-36.842	4	0	1	-.039	4	0	1
103	14	max	567.7	1	0	1	-.146	12	0	1	0	12	0	1
104		min	-69.416	3	0	1	-36.898	4	0	1	-.042	4	0	1
105	15	max	567.765	1	0	1	-.146	12	0	1	0	12	0	1
106		min	-69.367	3	0	1	-36.954	4	0	1	-.046	4	0	1
107	16	max	567.829	1	0	1	-.146	12	0	1	0	12	0	1
108		min	-69.319	3	0	1	-37.011	4	0	1	-.049	4	0	1
109	17	max	567.894	1	0	1	-.146	12	0	1	0	12	0	1
110		min	-69.27	3	0	1	-37.067	4	0	1	-.052	4	0	1
111	18	max	567.959	1	0	1	-.146	12	0	1	0	12	0	1
112		min	-69.222	3	0	1	-37.123	4	0	1	-.056	4	0	1
113	19	max	568.023	1	0	1	-.146	12	0	1	0	12	0	1
114		min	-69.173	3	0	1	-37.179	4	0	1	-.059	4	0	1
115	M6	1	max	1320.16	1	.625	6	1.116	4	0	0	4	0	1
116		min	-1200.777	3	.143	15	-.112	3	0	5	0	2	0	1
117	2	max	1320.266	1	.584	6	1.02	4	0	1	0	4	0	15
118		min	-1200.698	3	.134	15	-.112	3	0	5	0	2	0	6
119	3	max	1320.373	1	.543	6	.923	4	0	1	0	4	0	15
120		min	-1200.618	3	.124	15	-.112	3	0	5	0	2	0	6
121	4	max	1320.48	1	.502	6	.827	4	0	1	0	4	0	15
122		min	-1200.538	3	.114	15	-.112	3	0	5	0	12	0	6
123	5	max	1320.586	1	.46	6	.73	4	0	1	0	4	0	15
124		min	-1200.458	3	.105	15	-.112	3	0	5	0	3	0	6
125	6	max	1320.693	1	.419	6	.634	4	0	1	0	4	0	15
126		min	-1200.378	3	.095	15	-.112	3	0	5	0	3	0	6
127	7	max	1320.799	1	.379	2	.537	4	0	1	0	4	0	15
128		min	-1200.298	3	.085	15	-.112	3	0	5	0	3	0	6
129	8	max	1320.906	1	.347	2	.441	4	0	1	0	4	0	15
130		min	-1200.218	3	.075	15	-.112	3	0	5	0	3	0	6
131	9	max	1321.012	1	.315	2	.372	14	0	1	0	4	0	15
132		min	-1200.138	3	.066	15	-.112	3	0	5	0	3	0	6
133	10	max	1321.119	1	.283	2	.323	14	0	1	.001	4	0	15
134		min	-1200.058	3	.056	15	-.112	3	0	5	0	3	0	6
135	11	max	1321.225	1	.251	2	.288	1	0	1	.001	4	0	15
136		min	-1199.978	3	.046	15	-.112	3	0	5	0	3	0	6
137	12	max	1321.332	1	.218	2	.288	1	0	1	.001	4	0	15
138		min	-1199.899	3	.036	12	-.112	3	0	5	0	3	0	6
139	13	max	1321.438	1	.186	2	.288	1	0	1	.001	4	0	15
140		min	-1199.819	3	.02	12	-.151	5	0	5	0	3	0	2
141	14	max	1321.545	1	.154	2	.288	1	0	1	.001	4	0	15
142		min	-1199.739	3	-.001	3	-.247	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1321.652	1	.122	2	.288	1	0	1	.001	4	0	15
144		min	-1199.659	3	-.025	3	-.343	5	0	5	0	3	0	2
145	16	max	1321.758	1	.09	2	.288	1	0	1	0	4	0	15
146		min	-1199.579	3	-.05	3	-.44	5	0	5	0	3	0	2
147	17	max	1321.865	1	.058	2	.288	1	0	1	0	14	0	15
148		min	-1199.499	3	-.074	3	-.536	5	0	5	0	3	0	2
149	18	max	1321.971	1	.026	2	.288	1	0	1	0	14	0	15
150		min	-1199.419	3	-.098	3	-.633	5	0	5	0	3	0	2
151	19	max	1322.078	1	-.007	2	.288	1	0	1	0	14	0	15
152		min	-1199.339	3	-.122	3	-.729	5	0	5	0	3	0	2
153	M7	1	max	327.174	2	1.8	.018	1	0	2	0	4	0	2
154		min	-267.715	3	.427	15	-1.447	5	0	5	0	3	0	12
155	2	max	327.106	2	1.623	4	.018	1	0	2	0	4	0	2
156		min	-267.766	3	.386	15	-1.314	5	0	5	0	3	0	3
157	3	max	327.038	2	1.445	4	.018	1	0	2	0	4	0	2
158		min	-267.817	3	.344	15	-1.18	5	0	5	0	3	0	3
159	4	max	326.971	2	1.268	4	.018	1	0	2	0	2	0	2
160		min	-267.868	3	.302	15	-1.046	5	0	5	0	3	0	3
161	5	max	326.903	2	1.09	4	.018	1	0	2	0	2	0	15
162		min	-267.919	3	.26	15	-.913	5	0	5	0	5	0	6
163	6	max	326.835	2	.912	4	.018	1	0	2	0	2	0	15
164		min	-267.969	3	.219	15	-.779	5	0	5	0	5	0	6
165	7	max	326.767	2	.735	4	.018	1	0	2	0	2	0	15
166		min	-268.02	3	.177	15	-.645	5	0	5	0	5	0	6
167	8	max	326.699	2	.557	4	.018	1	0	2	0	2	0	15
168		min	-268.071	3	.135	15	-.512	5	0	5	0	5	0	6
169	9	max	326.631	2	.379	4	.018	1	0	2	0	2	0	15
170		min	-268.122	3	.093	15	-.378	5	0	5	0	5	-.001	6
171	10	max	326.563	2	.216	2	.018	1	0	2	0	2	0	15
172		min	-268.173	3	.043	12	-.245	5	0	5	0	5	-.001	6
173	11	max	326.495	2	.078	2	.018	1	0	2	0	2	0	15
174		min	-268.224	3	-.046	3	-.111	5	0	5	0	5	-.001	6
175	12	max	326.428	2	-.032	15	.029	14	0	2	0	2	0	15
176		min	-268.275	3	-.154	6	-.002	3	0	5	0	5	-.001	6
177	13	max	326.36	2	-.074	15	.161	4	0	2	0	2	0	15
178		min	-268.326	3	-.332	6	-.002	3	0	5	0	5	-.001	6
179	14	max	326.292	2	-.115	15	.294	4	0	2	0	2	0	15
180		min	-268.377	3	-.509	6	-.002	3	0	5	0	5	-.001	6
181	15	max	326.224	2	-.157	15	.428	4	0	2	0	2	0	15
182		min	-268.428	3	-.687	6	-.002	3	0	5	0	5	0	6
183	16	max	326.156	2	-.199	15	.562	4	0	2	0	2	0	15
184		min	-268.478	3	-.865	6	-.002	3	0	5	0	5	0	6
185	17	max	326.088	2	-.241	15	.695	4	0	2	0	2	0	15
186		min	-268.529	3	-1.042	6	-.002	3	0	5	0	5	0	6
187	18	max	326.02	2	-.283	15	.829	4	0	2	0	2	0	15
188		min	-268.58	3	-1.22	6	-.002	3	0	5	0	5	0	6
189	19	max	325.953	2	-.324	15	.963	4	0	2	0	2	0	1
190		min	-268.631	3	-1.398	6	-.002	3	0	5	0	5	0	1
191	M8	1	max	1618.622	1	0	.837	1	0	1	0	4	0	1
192		min	-257.179	3	0	1	-36.328	4	0	1	0	1	0	1
193	2	max	1618.686	1	0	1	.837	1	0	1	0	1	0	1
194		min	-257.13	3	0	1	-36.384	4	0	1	-.003	4	0	1
195	3	max	1618.751	1	0	1	.837	1	0	1	0	1	0	1
196		min	-257.082	3	0	1	-36.44	4	0	1	-.006	4	0	1
197	4	max	1618.816	1	0	1	.837	1	0	1	0	1	0	1
198		min	-257.033	3	0	1	-36.496	4	0	1	-.01	4	0	1
199	5	max	1618.881	1	0	1	.837	1	0	1	0	1	0	1





Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	418.088	1	.087	4	-.013	15	.001	1	.001	4	0	15
258			min	-353.692	3	-.019	1	-.191	1	-.002	5	0	1	0	4
259		16	max	418.195	1	.045	4	-.018	12	.001	1	.001	4	0	15
260			min	-353.612	3	-.051	1	-.191	1	-.002	5	0	1	0	4
261		17	max	418.301	1	.017	5	-.018	12	.001	1	.001	4	0	15
262			min	-353.532	3	-.083	1	-.216	4	-.002	5	0	1	0	4
263		18	max	418.408	1	.002	5	-.018	12	.001	1	.001	4	0	15
264			min	-353.452	3	-.115	1	-.313	4	-.002	5	0	1	0	4
265		19	max	418.514	1	-.008	15	-.018	12	.001	1	.001	4	0	15
266			min	-353.372	3	-.148	1	-.409	4	-.002	5	0	1	0	4
267	M11	1	max	59.801	2	1.788	6	.98	1	.002	4	.001	5	0	6
268			min	-108.584	9	.418	15	-1.139	5	0	10	-.002	1	0	15
269		2	max	59.733	2	1.611	6	.98	1	.002	4	.001	5	0	2
270			min	-108.64	9	.376	15	-1.005	5	0	10	-.002	1	0	15
271		3	max	59.666	2	1.433	6	.98	1	.002	4	.001	5	0	2
272			min	-108.697	9	.335	15	-.871	5	0	10	-.002	1	0	3
273		4	max	59.598	2	1.255	6	.98	1	.002	4	0	5	0	2
274			min	-108.753	9	.293	15	-.738	5	0	10	-.002	1	0	4
275		5	max	59.53	2	1.078	6	.98	1	.002	4	0	5	0	15
276			min	-108.81	9	.251	15	-.604	5	0	10	-.002	1	0	4
277		6	max	59.462	2	.9	6	.98	1	.002	4	0	5	0	15
278			min	-108.867	9	.209	15	-.47	5	0	10	-.001	1	0	4
279		7	max	59.394	2	.722	6	.98	1	.002	4	0	5	0	15
280			min	-108.923	9	.168	15	-.337	5	0	10	-.001	1	0	4
281		8	max	59.326	2	.545	6	.98	1	.002	4	0	5	0	15
282			min	-108.98	9	.126	15	-.203	5	0	10	-.001	1	-.001	4
283		9	max	59.258	2	.367	6	.98	1	.002	4	0	5	0	15
284			min	-109.036	9	.084	15	-.07	5	0	10	0	1	-.001	4
285		10	max	59.191	2	.189	6	.98	1	.002	4	0	5	0	15
286			min	-109.093	9	.042	15	.023	12	0	10	0	1	-.001	4
287		11	max	59.123	2	.05	2	.98	1	.002	4	0	5	0	15
288			min	-109.149	9	-.021	3	.023	12	0	10	0	1	-.001	4
289		12	max	59.055	2	-.041	15	.98	1	.002	4	0	5	0	15
290			min	-109.206	9	-.166	4	.023	12	0	10	0	1	-.001	4
291		13	max	58.987	2	-.083	15	.98	1	.002	4	0	5	0	15
292			min	-109.262	9	-.344	4	.023	12	0	10	0	2	-.001	4
293		14	max	58.919	2	-.125	15	.98	1	.002	4	0	4	0	15
294			min	-109.319	9	-.522	4	.023	12	0	10	0	10	-.001	4
295		15	max	58.851	2	-.166	15	.98	1	.002	4	0	4	0	15
296			min	-109.375	9	-.699	4	.023	12	0	10	0	10	0	4
297		16	max	58.783	2	-.208	15	1.06	4	.002	4	.001	4	0	15
298			min	-109.432	9	-.877	4	.023	12	0	10	0	10	0	4
299		17	max	58.715	2	-.25	15	1.194	4	.002	4	.001	4	0	15
300			min	-109.489	9	-1.055	4	.023	12	0	10	0	10	0	4
301		18	max	58.648	2	-.292	15	1.327	4	.002	4	.002	4	0	15
302			min	-109.545	9	-1.232	4	.023	12	0	10	0	10	0	4
303		19	max	58.58	2	-.333	15	1.461	4	.002	4	.002	4	0	1
304			min	-109.602	9	-1.41	4	.023	12	0	10	0	10	0	1
305	M12	1	max	566.634	1	0	1	4.52	1	0	1	0	4	0	1
306			min	-69.595	3	0	1	-33.148	5	0	1	0	3	0	1
307		2	max	566.699	1	0	1	4.52	1	0	1	0	1	0	1
308			min	-69.546	3	0	1	-33.204	5	0	1	-.003	5	0	1
309		3	max	566.763	1	0	1	4.52	1	0	1	0	1	0	1
310			min	-69.498	3	0	1	-33.261	5	0	1	-.006	5	0	1
311		4	max	566.828	1	0	1	4.52	1	0	1	.001	1	0	1
312			min	-69.449	3	0	1	-33.317	5	0	1	-.009	5	0	1
313		5	max	566.893	1	0	1	4.52	1	0	1	.002	1	0	1









Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	88.976	1	71.542	1	10.178	1	.01	3	-.001	15	.534	3
486			min	3.369	12	-61.808	3	.635	10	-.014	1	-.116	1	-.622	1
487		16	max	88.976	1	39.795	3	43.585	1	.01	3	.006	5	.545	3
488			min	3.369	12	-46.513	1	1.861	12	-.014	1	-.09	1	-.633	1
489		17	max	88.976	1	141.397	3	76.992	1	.01	3	.015	5	.459	3
490			min	3.369	12	-164.567	1	2.948	12	-.014	1	-.033	1	-.534	1
491		18	max	88.976	1	243	3	110.399	1	.01	3	.055	1	.277	3
492			min	3.369	12	-282.622	1	4.034	12	-.014	1	.003	12	-.323	1
493		19	max	88.976	1	344.602	3	143.806	1	.01	3	.175	1	0	1
494			min	3.369	12	-400.676	1	5.12	12	-.014	1	.007	12	0	3
495	M16	1	max	62.737	5	451.592	1	1.949	5	.007	3	.171	1	0	1
496			min	-88.094	1	-154.959	3	-142.937	1	-.015	1	-.036	5	0	3
497		2	max	53.385	5	318.526	1	3.671	5	.007	3	.052	1	.125	3
498			min	-88.094	1	-109.399	3	-109.53	1	-.015	1	-.033	5	-.364	1
499		3	max	44.034	5	185.46	1	5.393	5	.007	3	-.001	12	.207	3
500			min	-88.094	1	-63.839	3	-76.123	1	-.015	1	-.036	4	-.602	1
501		4	max	34.682	5	52.394	1	7.115	5	.007	3	-.003	12	.245	3
502			min	-88.094	1	-18.279	3	-42.716	1	-.015	1	-.092	1	-.714	1
503		5	max	25.331	5	27.281	3	8.837	5	.007	3	-.004	12	.241	3
504			min	-88.094	1	-80.672	1	-9.309	1	-.015	1	-.116	1	-.701	1
505		6	max	15.979	5	72.841	3	24.098	1	.007	3	-.004	12	.194	3
506			min	-88.094	1	-213.738	1	.528	12	-.015	1	-.109	1	-.562	1
507		7	max	6.628	5	118.401	3	57.505	1	.007	3	.004	5	.104	3
508			min	-88.094	1	-346.804	1	1.614	12	-.015	1	-.071	1	-.297	1
509		8	max	-1.732	15	163.961	3	90.912	1	.007	3	.017	4	.094	1
510			min	-88.094	1	-479.87	1	2.7	12	-.015	1	-.002	3	-.03	3
511		9	max	-1.88	12	209.522	3	124.32	1	.007	3	.101	1	.61	1
512			min	-88.094	1	-612.936	1	3.786	12	-.015	1	.002	12	-.206	3
513		10	max	35.336	5	-17.144	15	157.727	1	.005	14	.235	1	1.251	1
514			min	-91.033	1	-746.002	1	-7.461	3	-.015	1	.007	12	-.426	3
515		11	max	25.984	5	612.936	1	.836	5	.015	1	.102	1	.61	1
516			min	-91.033	1	-209.522	3	-124.064	1	-.007	3	-.017	5	-.206	3
517		12	max	16.633	5	479.87	1	2.558	5	.015	1	0	2	.094	1
518			min	-91.033	1	-163.961	3	-90.657	1	-.007	3	-.015	4	-.03	3
519		13	max	7.281	5	346.804	1	4.28	5	.015	1	-.002	12	.104	3
520			min	-91.033	1	-118.401	3	-57.249	1	-.007	3	-.07	1	-.297	1
521		14	max	-1.265	15	213.738	1	6.003	5	.015	1	-.003	12	.194	3
522			min	-91.033	1	-72.841	3	-23.842	1	-.007	3	-.108	1	-.562	1
523		15	max	-3.572	12	80.672	1	9.855	4	.015	1	0	15	.241	3
524			min	-91.033	1	-27.281	3	.444	12	-.007	3	-.115	1	-.701	1
525		16	max	-3.572	12	18.279	3	42.972	1	.015	1	.008	5	.245	3
526			min	-91.033	1	-52.394	1	1.53	12	-.007	3	-.09	1	-.714	1
527		17	max	-3.572	12	63.839	3	76.379	1	.015	1	.018	5	.207	3
528			min	-91.033	1	-185.46	1	2.616	12	-.007	3	-.033	1	-.602	1
529		18	max	-3.572	12	109.399	3	109.786	1	.015	1	.054	1	.125	3
530			min	-91.033	1	-318.526	1	3.702	12	-.007	3	.002	12	-.364	1
531		19	max	-3.572	12	154.959	3	143.193	1	.015	1	.174	1	0	1
532			min	-91.033	1	-451.592	1	4.789	12	-.007	3	.006	12	0	5
533	M15	1	max	0	4	2.305	1	.023	3	0	1	0	1	0	1
534			min	-35.153	1	0	4	-.028	1	0	3	0	3	0	1
535		2	max	0	4	2.049	1	.023	3	0	1	0	1	0	4
536			min	-35.233	1	0	4	-.028	1	0	3	0	3	-.001	1
537		3	max	0	4	1.793	1	.023	3	0	1	0	1	0	4
538			min	-35.312	1	0	4	-.028	1	0	3	0	3	-.002	1
539		4	max	0	4	1.537	1	.023	3	0	1	0	1	0	4
540			min	-35.392	1	0	4	-.028	1	0	3	0	3	-.003	1
541		5	max	0	4	1.281	1	.023	3	0	1	0	1	0	4



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-35.472	1	0	4	-.028	1	0	3	0	3	-.004	1
543		6	max	0	4	1.025	1	.023	3	0	1	0	1	0	4
544			min	-35.551	1	0	4	-.028	1	0	3	0	3	-.004	1
545		7	max	0	4	.768	1	.023	3	0	1	0	3	0	4
546			min	-35.631	1	0	4	-.028	1	0	3	0	2	-.005	1
547		8	max	0	4	.512	1	.023	3	0	1	0	3	0	4
548			min	-35.71	1	0	4	-.028	1	0	3	0	1	-.005	1
549		9	max	0	4	.256	1	.023	3	0	1	0	3	0	4
550			min	-35.79	1	0	4	-.028	1	0	3	0	1	-.005	1
551		10	max	0	4	0	1	.023	3	0	1	0	3	0	4
552			min	-35.869	1	0	1	-.028	1	0	3	0	1	-.005	1
553		11	max	0	4	0	4	.023	3	0	1	0	3	0	4
554			min	-35.949	1	-.256	2	-.028	1	0	3	0	1	-.005	1
555		12	max	0	4	0	4	.023	3	0	1	0	3	0	4
556			min	-36.029	1	-.512	2	-.028	1	0	3	0	1	-.005	1
557		13	max	0	4	0	4	.023	3	0	1	0	3	0	4
558			min	-36.108	1	-.768	2	-.028	1	0	3	0	1	-.005	1
559		14	max	0	4	0	4	.023	3	0	1	0	3	0	4
560			min	-36.188	1	-1.025	2	-.028	1	0	3	0	1	-.004	1
561		15	max	0	4	0	4	.023	3	0	1	0	3	0	4
562			min	-36.267	1	-1.281	2	-.028	1	0	3	0	1	-.004	1
563		16	max	0	4	0	4	.023	3	0	1	0	3	0	4
564			min	-36.347	1	-1.537	2	-.028	1	0	3	0	1	-.003	1
565		17	max	0	4	0	4	.023	3	0	1	0	3	0	4
566			min	-36.426	1	-1.793	2	-.028	1	0	3	0	1	-.002	1
567		18	max	0	4	0	4	.023	3	0	1	0	3	0	4
568			min	-36.506	1	-2.049	2	-.028	1	0	3	0	1	-.001	1
569		19	max	0	4	0	4	.023	3	0	1	0	3	0	1
570			min	-36.585	1	-2.305	2	-.028	1	0	3	0	1	0	1
571	M16A	1	max	-.917	10	3.576	4	.225	4	0	3	0	3	0	1
572			min	-271.21	4	1.111	15	-.009	3	0	1	0	4	0	1
573		2	max	-.851	10	3.179	4	.203	4	0	3	0	3	0	15
574			min	-271.329	4	.988	15	-.009	3	0	1	0	4	-.002	4
575		3	max	-.785	10	2.781	4	.181	4	0	3	0	3	0	15
576			min	-271.448	4	.864	15	-.009	3	0	1	0	4	-.003	4
577		4	max	-.718	10	2.384	4	.159	4	0	3	0	3	-.001	15
578			min	-271.567	4	.741	15	-.009	3	0	1	0	4	-.004	4
579		5	max	-.652	10	1.987	4	.138	4	0	3	0	3	-.002	15
580			min	-271.686	4	.617	15	-.009	3	0	1	0	1	-.006	4
581		6	max	-.586	10	1.589	4	.116	4	0	3	0	5	-.002	15
582			min	-271.805	4	.494	15	-.009	3	0	1	0	1	-.006	4
583		7	max	-.519	10	1.192	4	.094	4	0	3	0	5	-.002	15
584			min	-271.923	4	.37	15	-.009	3	0	1	0	1	-.007	4
585		8	max	-.453	10	.795	4	.072	4	0	3	0	5	-.002	15
586			min	-272.042	4	.247	15	-.009	3	0	1	0	1	-.008	4
587		9	max	-.387	10	.397	4	.05	4	0	3	0	5	-.002	15
588			min	-272.161	4	.123	15	-.009	3	0	1	0	1	-.008	4
589		10	max	-.321	10	0	1	.029	4	0	3	0	5	-.002	15
590			min	-272.28	4	0	1	-.009	3	0	1	0	1	-.008	4
591		11	max	-.254	10	-.123	15	.02	1	0	3	0	5	-.002	15
592			min	-272.399	4	-.397	4	-.009	3	0	1	0	1	-.008	4
593		12	max	-.188	10	-.247	15	.02	1	0	3	0	5	-.002	15
594			min	-272.518	4	-.795	4	-.019	5	0	1	0	1	-.008	4
595		13	max	-.122	10	-.37	15	.02	1	0	3	0	5	-.002	15
596			min	-272.637	4	-1.192	4	-.04	5	0	1	0	3	-.007	4
597		14	max	-.055	10	-.494	15	.02	1	0	3	0	4	-.002	15
598			min	-272.756	4	-1.589	4	-.062	5	0	1	0	3	-.006	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.011	10	-6.17	15	.02	1	0	3	0	4	-.002	15
600		min	-272.875	4	-1.987	4	-.084	5	0	1	0	3	-.006	4
601	16	max	.077	10	-7.41	15	.02	1	0	3	0	4	-.001	15
602		min	-272.994	4	-2.384	4	-.106	5	0	1	0	3	-.004	4
603	17	max	.144	10	-.864	15	.02	1	0	3	0	1	0	15
604		min	-273.112	4	-2.781	4	-.128	5	0	1	0	3	-.003	4
605	18	max	.21	10	-.988	15	.02	1	0	3	0	1	0	15
606		min	-273.231	4	-3.179	4	-.149	5	0	1	0	5	-.002	4
607	19	max	.276	10	-1.111	15	.02	1	0	3	0	1	0	1
608		min	-273.35	4	-3.576	4	-.171	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.008	2	.016	1	2.04e-3	5	NC	3	NC	3	
2			min	-.003	3	-.006	3	-.02	5	-1.345e-3	1	4388.4	2	2023.301	1	
3			2	max	.003	1	.007	2	.015	1	2.068e-3	5	NC	3	NC	3
4				min	-.003	3	-.006	3	-.019	5	-1.29e-3	1	4756.155	2	2189.494	1
5			3	max	.003	1	.006	2	.014	1	2.095e-3	5	NC	3	NC	3
6				min	-.003	3	-.006	3	-.019	5	-1.235e-3	1	5187.869	2	2385.177	1
7			4	max	.003	1	.006	2	.013	1	2.123e-3	5	NC	3	NC	3
8				min	-.002	3	-.006	3	-.018	5	-1.18e-3	1	5698.011	2	2617.5	1
9			5	max	.003	1	.005	2	.011	1	2.151e-3	5	NC	3	NC	3
10				min	-.002	3	-.005	3	-.017	5	-1.124e-3	1	6305.599	2	2896.003	1
11			6	max	.002	1	.005	2	.01	1	2.178e-3	5	NC	1	NC	3
12				min	-.002	3	-.005	3	-.016	5	-1.069e-3	1	7036.028	2	3233.647	1
13			7	max	.002	1	.004	2	.009	1	2.206e-3	5	NC	1	NC	3
14				min	-.002	3	-.005	3	-.015	5	-1.014e-3	1	7923.844	2	3648.412	1
15			8	max	.002	1	.004	2	.008	1	2.234e-3	5	NC	1	NC	2
16				min	-.002	3	-.005	3	-.014	5	-9.583e-4	1	9017.072	2	4165.843	1
17			9	max	.002	1	.003	2	.007	1	2.261e-3	5	NC	1	NC	2
18				min	-.002	3	-.004	3	-.013	5	-9.03e-4	1	NC	1	4823.261	1
19			10	max	.002	1	.003	2	.006	1	2.289e-3	5	NC	1	NC	2
20				min	-.001	3	-.004	3	-.012	5	-8.478e-4	1	NC	1	5677.022	1
21		11	max	.001	1	.002	2	.005	1	2.317e-3	5	NC	1	NC	2	
22			min	-.001	3	-.004	3	-.01	5	-7.925e-4	1	NC	1	6815.648	1	
23		12	max	.001	1	.002	2	.004	1	2.344e-3	5	NC	1	NC	2	
24			min	-.001	3	-.003	3	-.009	5	-7.372e-4	1	NC	1	8385.075	1	
25		13	max	.001	1	.002	2	.003	1	2.372e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.008	5	-6.819e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.399e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.007	5	-6.266e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.427e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.005	5	-5.713e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	2.455e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.004	5	-5.16e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.482e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.003	5	-4.607e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.51e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-4.054e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.538e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.501e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.609e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.167e-3	5	NC	1	NC	1	
41		2	max	0	9	0	2	.006	5	2.032e-4	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-1.175e-3	5	NC	1	NC	1	



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	9	0	2	.012	5	2.455e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	-.001	1	-1.182e-3	5	NC	1	8040.159	14
45		4	max	0	9	0	2	.018	5	2.878e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	-.001	1	-1.19e-3	5	NC	1	5241.313	14
47		5	max	0	9	0	2	.025	5	3.301e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	-1.198e-3	5	NC	1	3854.555	14
49		6	max	0	9	0	2	.031	4	3.725e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.206e-3	5	NC	1	3031.274	14
51		7	max	0	9	.001	2	.037	4	4.148e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	-.001	1	-1.214e-3	5	NC	1	2488.796	14
53		8	max	0	9	.001	2	.043	4	4.571e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-1.221e-3	5	NC	1	2106.105	14
55		9	max	0	9	.002	2	.05	4	4.994e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	2	-1.229e-3	5	NC	1	1822.781	14
57		10	max	0	9	.002	2	.056	4	5.417e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-1.237e-3	5	NC	1	1605.323	14
59		11	max	0	9	.003	2	.062	4	5.84e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-1.245e-3	5	NC	1	1433.672	14
61		12	max	0	9	.003	2	.068	4	6.263e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	12	-1.253e-3	5	NC	1	1295.099	14
63		13	max	0	9	.004	2	.074	4	6.686e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	12	-1.26e-3	5	NC	1	1181.138	14
65		14	max	0	9	.005	2	.08	4	7.109e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	12	-1.268e-3	5	9439.052	2	1085.947	14
67		15	max	0	9	.006	2	.086	4	7.532e-4	1	NC	3	NC	2
68			min	0	2	-.007	3	0	12	-1.276e-3	5	7983.097	2	1005.366	14
69		16	max	.001	9	.007	2	.092	4	7.955e-4	1	NC	3	NC	2
70			min	0	2	-.008	3	0	12	-1.284e-3	5	6845.188	2	936.35	14
71		17	max	.001	9	.008	2	.098	4	8.378e-4	1	NC	3	NC	2
72			min	0	2	-.008	3	0	12	-1.292e-3	5	5947.645	2	876.626	14
73		18	max	.001	9	.009	2	.103	4	8.801e-4	1	NC	3	NC	2
74			min	0	2	-.008	3	0	12	-1.299e-3	5	5234.021	2	824.456	14
75		19	max	.001	9	.01	2	.109	4	9.224e-4	1	NC	3	NC	2
76			min	0	2	-.008	3	0	12	-1.307e-3	5	4663.081	2	778.492	14
77	M4	1	max	.003	1	.009	2	0	12	5.613e-3	5	NC	1	NC	3
78			min	0	3	-.007	3	-.115	4	-1.115e-3	1	NC	1	167.555	4
79		2	max	.003	1	.008	2	0	12	5.613e-3	5	NC	1	NC	3
80			min	0	3	-.006	3	-.106	4	-1.115e-3	1	NC	1	182.662	4
81		3	max	.002	1	.008	2	0	12	5.613e-3	5	NC	1	NC	3
82			min	0	3	-.006	3	-.096	4	-1.115e-3	1	NC	1	200.644	4
83		4	max	.002	1	.007	2	0	12	5.613e-3	5	NC	1	NC	2
84			min	0	3	-.006	3	-.087	4	-1.115e-3	1	NC	1	222.259	4
85		5	max	.002	1	.007	2	0	12	5.613e-3	5	NC	1	NC	2
86			min	0	3	-.005	3	-.078	4	-1.115e-3	1	NC	1	248.536	4
87		6	max	.002	1	.006	2	0	12	5.613e-3	5	NC	1	NC	2
88			min	0	3	-.005	3	-.069	4	-1.115e-3	1	NC	1	280.912	4
89		7	max	.002	1	.006	2	0	12	5.613e-3	5	NC	1	NC	2
90			min	0	3	-.004	3	-.06	4	-1.115e-3	1	NC	1	321.43	4
91		8	max	.002	1	.005	2	0	12	5.613e-3	5	NC	1	NC	2
92			min	0	3	-.004	3	-.052	4	-1.115e-3	1	NC	1	373.08	4
93		9	max	.002	1	.005	2	0	12	5.613e-3	5	NC	1	NC	2
94			min	0	3	-.004	3	-.044	4	-1.115e-3	1	NC	1	440.39	4
95		10	max	.001	1	.004	2	0	12	5.613e-3	5	NC	1	NC	2
96			min	0	3	-.003	3	-.036	4	-1.115e-3	1	NC	1	530.472	4
97		11	max	.001	1	.004	2	0	12	5.613e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.03	4	-1.115e-3	1	NC	1	655.043	4
99		12	max	.001	1	.003	2	0	12	5.613e-3	5	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.013	4	1.536e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-1.202e-3	4	NC	1	NC	1
159		4	max	0	3	.004	2	.019	4	1.667e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	2	-1.191e-3	4	NC	1	NC	1
161		5	max	0	3	.006	2	.026	4	1.798e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	2	-1.18e-3	4	8301.046	2	NC	1
163		6	max	0	3	.007	2	.032	4	1.929e-5	1	NC	3	NC	1
164			min	-.001	2	-.008	3	0	2	-1.169e-3	4	6651.519	2	NC	1
165		7	max	.001	3	.008	2	.039	4	2.115e-5	3	NC	3	NC	1
166			min	-.001	2	-.009	3	0	2	-1.158e-3	4	5524.683	2	NC	1
167		8	max	.001	3	.01	2	.045	4	3.171e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-1.147e-3	4	4699.497	2	NC	1
169		9	max	.001	3	.011	2	.051	4	4.227e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-1.136e-3	4	4065.794	2	NC	1
171		10	max	.002	3	.013	2	.058	4	5.282e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	1	-1.125e-3	4	3562.497	2	NC	1
173		11	max	.002	3	.015	2	.064	4	6.338e-5	3	NC	3	NC	1
174			min	-.002	2	-.015	3	-.001	1	-1.114e-3	4	3152.928	2	NC	1
175		12	max	.002	3	.016	2	.07	4	7.393e-5	3	NC	3	NC	1
176			min	-.002	2	-.016	3	-.001	1	-1.104e-3	4	2813.62	2	NC	1
177		13	max	.002	3	.018	2	.076	4	8.449e-5	3	NC	3	NC	1
178			min	-.002	2	-.017	3	-.001	1	-1.093e-3	4	2528.757	2	NC	1
179		14	max	.002	3	.02	2	.082	4	9.504e-5	3	NC	3	NC	1
180			min	-.003	2	-.018	3	-.002	1	-1.082e-3	4	2287.198	2	NC	1
181		15	max	.002	3	.022	2	.087	4	1.056e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	-.002	1	-1.071e-3	4	2080.808	2	NC	1
183		16	max	.003	3	.024	2	.093	4	1.162e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	-.002	1	-1.06e-3	4	1903.459	2	NC	1
185		17	max	.003	3	.026	2	.099	4	1.267e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	-.002	1	-1.049e-3	4	1750.425	2	NC	1
187		18	max	.003	3	.028	2	.104	4	1.373e-4	3	NC	3	NC	1
188			min	-.004	2	-.021	3	-.002	1	-1.038e-3	4	1617.984	2	NC	1
189		19	max	.003	3	.031	2	.11	4	1.478e-4	3	NC	3	NC	1
190			min	-.004	2	-.022	3	-.002	1	-1.027e-3	4	1503.167	2	NC	1
191	M8	1	max	.008	1	.028	2	.003	1	5.375e-3	4	NC	1	NC	2
192			min	-.001	3	-.019	3	-.116	4	-1.609e-4	1	NC	1	166.855	4
193		2	max	.007	1	.027	2	.002	1	5.375e-3	4	NC	1	NC	2
194			min	-.001	3	-.018	3	-.106	4	-1.609e-4	1	NC	1	181.899	4
195		3	max	.007	1	.025	2	.002	1	5.375e-3	4	NC	1	NC	2
196			min	-.001	3	-.017	3	-.097	4	-1.609e-4	1	NC	1	199.805	4
197		4	max	.006	1	.024	2	.002	1	5.375e-3	4	NC	1	NC	2
198			min	-.001	3	-.016	3	-.087	4	-1.609e-4	1	NC	1	221.328	4
199		5	max	.006	1	.022	2	.002	1	5.375e-3	4	NC	1	NC	1
200			min	0	3	-.015	3	-.078	4	-1.609e-4	1	NC	1	247.495	4
201		6	max	.006	1	.021	2	.002	1	5.375e-3	4	NC	1	NC	1
202			min	0	3	-.014	3	-.069	4	-1.609e-4	1	NC	1	279.735	4
203		7	max	.005	1	.019	2	.001	1	5.375e-3	4	NC	1	NC	1
204			min	0	3	-.013	3	-.06	4	-1.609e-4	1	NC	1	320.082	4
205		8	max	.005	1	.017	2	.001	1	5.375e-3	4	NC	1	NC	1
206			min	0	3	-.012	3	-.052	4	-1.609e-4	1	NC	1	371.515	4
207		9	max	.004	1	.016	2	.001	1	5.375e-3	4	NC	1	NC	1
208			min	0	3	-.011	3	-.044	4	-1.609e-4	1	NC	1	438.541	4
209		10	max	.004	1	.014	2	0	1	5.375e-3	4	NC	1	NC	1
210			min	0	3	-.01	3	-.037	4	-1.609e-4	1	NC	1	528.244	4
211		11	max	.003	1	.013	2	0	1	5.375e-3	4	NC	1	NC	1
212			min	0	3	-.009	3	-.03	4	-1.609e-4	1	NC	1	652.291	4
213		12	max	.003	1	.011	2	0	1	5.375e-3	4	NC	1	NC	1







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.021	5	9.406e-5	10	NC	1	909.819	5
329		max	0	1	.003	2	.002	1	7.047e-3	4	NC	1	NC	2
330		min	0	3	-.002	3	-.016	5	9.406e-5	10	NC	1	1207.155	5
331		max	0	1	.002	2	.002	1	7.047e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.011	5	9.406e-5	10	NC	1	1692.354	5
333		max	0	1	.002	2	.001	1	7.047e-3	4	NC	1	NC	1
334		min	0	3	-.001	3	-.008	5	9.406e-5	10	NC	1	2567.93	5
335		max	0	1	.001	2	0	1	7.047e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.004	5	9.406e-5	10	NC	1	4409.594	5
337		max	0	1	0	2	0	1	7.047e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.002	5	9.406e-5	10	NC	1	9453.287	5
339		max	0	1	0	2	0	1	7.047e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	9.406e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	7.047e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	9.406e-5	10	NC	1	NC	1
343	M1	max	.006	3	.023	3	.011	5	1.996e-2	1	NC	1	NC	1
344		min	-.007	2	-.03	1	-.005	1	-1.708e-2	3	NC	1	NC	1
345		max	.006	3	.013	3	.015	5	9.51e-3	1	NC	4	NC	2
346		min	-.007	2	-.016	1	-.012	1	-8.456e-3	3	3340.165	1	6822.313	1
347		max	.006	3	.003	3	.02	5	6.035e-4	5	NC	4	NC	2
348		min	-.007	2	-.003	1	-.016	1	-7.453e-4	1	1727.586	1	4135.471	1
349		max	.006	3	.008	1	.026	5	6.07e-4	5	NC	5	NC	3
350		min	-.007	2	-.005	3	-.019	1	-6.206e-4	1	1222.404	1	3135.205	5
351		max	.006	3	.017	1	.032	5	6.105e-4	5	NC	5	NC	3
352		min	-.007	2	-.012	3	-.019	1	-4.959e-4	1	979.677	1	2249.958	5
353		max	.006	3	.025	1	.038	5	6.139e-4	5	NC	5	NC	3
354		min	-.007	2	-.017	3	-.018	1	-3.712e-4	1	842.57	1	1732.242	5
355		max	.006	3	.031	1	.045	5	6.174e-4	5	NC	5	NC	2
356		min	-.007	2	-.021	3	-.016	1	-2.464e-4	1	759.651	1	1396.331	5
357		max	.006	3	.035	1	.051	5	6.208e-4	5	NC	5	NC	2
358		min	-.007	2	-.023	3	-.013	1	-1.217e-4	1	709.539	1	1163.046	5
359		max	.006	3	.037	1	.058	5	6.243e-4	5	NC	5	NC	1
360		min	-.008	2	-.025	3	-.009	1	-9.087e-6	2	682.257	1	987.125	4
361		max	.006	3	.038	1	.065	5	6.448e-4	4	NC	5	NC	1
362		min	-.008	2	-.025	3	-.005	1	1.243e-5	10	673.211	1	845.414	4
363		max	.006	3	.037	1	.073	4	6.755e-4	4	NC	5	NC	1
364		min	-.008	2	-.024	3	-.001	1	2.317e-5	10	681.018	1	738.688	4
365		max	.006	3	.035	1	.081	4	7.063e-4	4	NC	5	NC	2
366		min	-.008	2	-.022	3	0	10	2.875e-5	12	706.929	1	656.494	4
367		max	.006	3	.03	1	.088	4	7.37e-4	4	NC	5	NC	2
368		min	-.008	2	-.019	3	0	12	3.181e-5	12	755.36	1	592.12	4
369		max	.006	3	.024	1	.096	4	7.678e-4	4	NC	5	NC	3
370		min	-.008	2	-.015	3	0	12	3.488e-5	12	835.976	1	541.097	4
371		max	.006	3	.016	1	.103	4	7.985e-4	4	NC	5	NC	3
372		min	-.008	2	-.01	3	0	12	3.795e-5	12	969.491	1	500.364	4
373		max	.006	3	.007	1	.109	4	1.185e-3	4	NC	5	NC	3
374		min	-.008	2	-.004	3	0	12	4.001e-5	12	1205.294	1	467.772	4
375		max	.006	3	.002	3	.115	4	1.003e-2	4	NC	4	NC	2
376		min	-.008	2	-.005	1	0	12	1.31e-5	10	1691.764	1	441.823	4
377		max	.006	3	.01	3	.12	4	1.119e-2	1	NC	4	NC	2
378		min	-.008	2	-.018	1	0	10	-3.871e-3	3	3260.856	1	421.389	4
379		max	.006	3	.018	3	.125	4	2.248e-2	1	NC	1	NC	1
380		min	-.008	2	-.033	1	-.004	1	-7.844e-3	3	NC	1	406.154	4
381	M5	max	.018	3	.069	3	.01	5	4.903e-6	4	NC	1	NC	1
382		min	-.024	2	-.092	1	-.006	1	5.655e-8	10	NC	1	NC	1
383		max	.018	3	.039	3	.015	5	2.965e-4	5	NC	5	NC	1
384		min	-.024	2	-.05	1	-.005	1	-9.154e-5	1	1115.297	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.018	3	.01	3	.021	5	5.831e-4	5	NC	5	NC	1
386		min	-.024	2	-.011	1	-.005	1	-1.818e-4	1	574.191	1	NC	1
387	4	max	.018	3	.022	1	.026	5	6.048e-4	5	NC	5	NC	1
388		min	-.024	2	-.013	3	-.004	1	-1.697e-4	1	405.219	1	NC	1
389	5	max	.018	3	.051	1	.033	5	6.264e-4	5	NC	15	NC	1
390		min	-.024	2	-.033	3	-.003	1	-1.575e-4	1	323.983	1	NC	1
391	6	max	.018	3	.074	1	.04	5	6.481e-4	5	NC	15	NC	1
392		min	-.024	2	-.048	3	-.003	1	-1.454e-4	1	278.014	1	NC	1
393	7	max	.018	3	.092	1	.047	5	6.697e-4	5	NC	15	NC	1
394		min	-.024	2	-.06	3	-.003	1	-1.333e-4	1	250.117	1	NC	1
395	8	max	.018	3	.105	1	.054	5	6.914e-4	5	9994.549	15	NC	1
396		min	-.024	2	-.067	3	-.002	1	-1.211e-4	1	233.139	1	NC	1
397	9	max	.018	3	.113	1	.061	5	7.13e-4	5	9638.134	15	NC	1
398		min	-.025	2	-.072	3	-.002	1	-1.09e-4	1	223.739	1	NC	1
399	10	max	.018	3	.116	1	.069	5	7.347e-4	5	9536.836	15	NC	1
400		min	-.025	2	-.072	3	-.002	1	-9.689e-5	1	220.368	1	NC	1
401	11	max	.018	3	.113	1	.076	5	7.563e-4	5	9673.141	15	NC	1
402		min	-.025	2	-.07	3	-.002	1	-8.476e-5	1	222.544	1	NC	1
403	12	max	.018	3	.106	1	.083	5	7.779e-4	5	NC	15	NC	1
404		min	-.025	2	-.064	3	-.002	1	-7.264e-5	1	230.658	1	NC	1
405	13	max	.018	3	.092	1	.091	4	7.996e-4	5	NC	15	NC	1
406		min	-.025	2	-.055	3	-.002	1	-6.051e-5	1	246.139	1	NC	1
407	14	max	.018	3	.074	1	.098	4	8.212e-4	5	NC	15	NC	1
408		min	-.025	2	-.043	3	-.002	1	-4.838e-5	1	272.146	1	9259.72	4
409	15	max	.018	3	.049	1	.104	4	8.429e-4	5	NC	15	NC	1
410		min	-.025	2	-.029	3	-.002	1	-3.903e-5	2	315.484	1	9113.521	4
411	16	max	.018	3	.02	1	.11	4	1.216e-3	5	NC	5	NC	1
412		min	-.025	2	-.012	3	-.002	1	-3.664e-5	2	392.51	1	9830.575	4
413	17	max	.018	3	.007	3	.116	4	1.003e-2	4	NC	5	NC	1
414		min	-.025	2	-.015	1	-.003	1	-2.418e-4	1	553.417	1	NC	1
415	18	max	.018	3	.029	3	.121	4	5.145e-3	4	NC	5	NC	1
416		min	-.025	2	-.057	1	-.003	1	-1.24e-4	1	1072.459	1	NC	1
417	19	max	.018	3	.051	3	.125	4	1.649e-6	5	NC	1	NC	1
418		min	-.025	2	-.101	1	-.003	1	-1.103e-7	3	NC	1	NC	1
419	M9	1	max	.006	.023	3	.009	5	1.708e-2	3	NC	1	NC	1
420		min	-.007	2	-.03	1	-.008	1	-1.996e-2	1	NC	1	NC	1
421	2	max	.006	3	.013	3	.008	5	8.464e-3	3	NC	4	NC	2
422		min	-.007	2	-.016	1	-.001	1	-9.817e-3	1	3340.948	1	8112.281	1
423	3	max	.006	3	.003	3	.009	4	1.364e-4	1	NC	5	NC	2
424		min	-.007	2	-.003	1	0	3	6.059e-6	12	1728.001	1	5060.959	1
425	4	max	.006	3	.008	1	.011	4	3.898e-5	5	NC	5	NC	3
426		min	-.007	2	-.005	3	0	3	-1.712e-7	3	1222.694	1	4309.198	1
427	5	max	.006	3	.017	1	.014	4	2.458e-5	5	NC	5	NC	3
428		min	-.007	2	-.012	3	0	3	-7.583e-5	1	979.897	1	4299.353	1
429	6	max	.006	3	.025	1	.018	4	1.017e-5	5	NC	5	NC	2
430		min	-.007	2	-.017	3	-.001	3	-1.819e-4	1	842.745	1	3869.3	14
431	7	max	.006	3	.031	1	.023	4	-2.531e-6	15	NC	5	NC	2
432		min	-.007	2	-.021	3	-.002	3	-2.881e-4	1	759.795	1	2787.812	4
433	8	max	.006	3	.035	1	.029	4	-1.213e-5	15	NC	5	NC	1
434		min	-.007	2	-.023	3	-.002	3	-3.942e-4	1	709.659	1	2087.483	4
435	9	max	.006	3	.037	1	.035	5	-2.173e-5	15	NC	5	NC	1
436		min	-.007	2	-.025	3	-.004	1	-5.003e-4	1	682.359	1	1628.296	4
437	10	max	.006	3	.038	1	.043	5	-3.133e-5	15	NC	5	NC	1
438		min	-.008	2	-.025	3	-.008	1	-6.064e-4	1	673.298	1	1310.944	4
439	11	max	.006	3	.037	1	.051	5	-4.093e-5	15	NC	5	NC	2
440		min	-.008	2	-.024	3	-.011	1	-7.125e-4	1	681.092	1	1082.371	4
441	12	max	.006	3	.035	1	.059	5	-4.826e-5	12	NC	5	NC	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.174	3	.149	1	7.999e-3	1	NC	5	NC	3
500		min	-.125	4	-.48	1	.009	10	-4.231e-3	3	456.295	1	1324.115	1
501	4	max	.002	1	.219	3	.223	1	9.239e-3	1	NC	15	NC	3
502		min	-.125	4	-.606	1	.014	10	-4.839e-3	3	356.031	1	891.535	1
503	5	max	.003	1	.233	3	.26	1	1.048e-2	1	NC	15	NC	12
504		min	-.125	4	-.642	1	.017	10	-5.447e-3	3	334.742	1	768.152	1
505	6	max	.003	1	.216	3	.248	1	1.172e-2	1	NC	5	NC	10
506		min	-.125	4	-.591	1	.014	10	-6.054e-3	3	365.525	1	803.708	1
507	7	max	.003	1	.176	3	.191	1	1.296e-2	1	NC	5	NC	5
508		min	-.125	4	-.469	1	.008	10	-6.662e-3	3	467.146	1	1037.861	1
509	8	max	.003	1	.123	3	.105	1	1.42e-2	1	NC	5	NC	3
510		min	-.125	4	-.312	1	0	10	-7.269e-3	3	731.534	1	1850.362	1
511	9	max	.003	1	.074	3	.021	3	1.544e-2	1	NC	5	NC	2
512		min	-.125	4	-.167	1	-.009	10	-7.877e-3	3	1519.434	1	8207.608	1
513	10	max	.003	1	.051	3	.018	3	1.668e-2	1	NC	4	NC	1
514		min	-.125	4	-.101	1	-.025	2	-8.485e-3	3	2977.857	1	NC	1
515	11	max	.003	1	.074	3	.02	14	1.544e-2	1	NC	5	NC	2
516		min	-.125	4	-.167	1	-.009	10	-7.877e-3	3	1519.434	1	8874.168	1
517	12	max	.003	1	.123	3	.101	1	1.42e-2	1	NC	5	NC	3
518		min	-.125	4	-.312	1	0	10	-7.269e-3	3	731.535	1	1911.177	1
519	13	max	.003	1	.176	3	.186	1	1.296e-2	1	NC	5	NC	3
520		min	-.125	4	-.469	1	.008	10	-6.661e-3	3	467.146	1	1063.953	1
521	14	max	.004	1	.216	3	.243	1	1.172e-2	1	NC	5	NC	3
522		min	-.125	4	-.591	1	.006	15	-6.053e-3	3	365.525	1	822.172	1
523	15	max	.004	1	.233	3	.254	1	1.048e-2	1	NC	15	NC	3
524		min	-.125	4	-.642	1	0	15	-5.445e-3	3	334.742	1	786.044	1
525	16	max	.004	1	.219	3	.218	1	9.241e-3	1	NC	15	NC	3
526		min	-.125	4	-.606	1	-.01	5	-4.837e-3	3	356.031	1	914.517	1
527	17	max	.004	1	.174	3	.144	1	8.001e-3	1	NC	5	NC	3
528		min	-.125	4	-.48	1	-.016	5	-4.229e-3	3	456.296	1	1365.903	1
529	18	max	.004	1	.104	3	.057	1	6.762e-3	1	NC	5	NC	3
530		min	-.125	4	-.279	1	-.014	5	-3.621e-3	3	829.283	1	3265.619	1
531	19	max	.004	1	.018	3	.006	3	5.522e-3	1	NC	1	NC	1
532		min	-.125	4	-.033	1	-.008	2	-3.013e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.166e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.342e-4	5	NC	1	NC	1
535	2	max	0	1	-.003	15	.014	4	8.209e-4	3	NC	5	NC	1
536		min	-.001	5	-.023	6	0	3	-8.074e-4	1	4537.219	6	7739.616	4
537	3	max	0	1	-.005	15	.03	4	1.325e-3	3	NC	5	NC	1
538		min	-.002	5	-.046	6	-.003	3	-1.54e-3	1	2308.836	6	3587.518	4
539	4	max	0	1	-.007	15	.046	4	1.829e-3	3	NC	15	NC	3
540		min	-.003	5	-.067	6	-.006	3	-2.273e-3	1	1583.996	6	2316.238	4
541	5	max	0	1	-.009	15	.061	4	2.334e-3	3	NC	15	NC	9
542		min	-.004	5	-.086	6	-.01	3	-3.006e-3	1	1236.008	6	1742.101	4
543	6	max	0	1	-.011	15	.074	4	2.838e-3	3	8797.406	15	9819.219	10
544		min	-.005	5	-.102	6	-.015	3	-3.739e-3	1	1040.232	6	1440.574	4
545	7	max	0	1	-.012	15	.084	4	3.342e-3	3	7801.712	15	7738.455	10
546		min	-.007	5	-.115	1	-.019	3	-4.472e-3	1	922.498	6	1276.369	4
547	8	max	0	1	-.013	15	.089	4	3.847e-3	3	7204.146	15	6420.344	10
548		min	-.008	5	-.125	1	-.024	3	-5.205e-3	1	851.84	6	1196.24	4
549	9	max	0	1	-.014	15	.09	4	4.351e-3	3	6882.5	15	5553.598	10
550		min	-.009	5	-.131	1	-.028	3	-5.938e-3	1	813.807	6	1178.757	4
551	10	max	0	1	-.014	15	.087	4	4.855e-3	3	6780.747	15	4980.515	10
552		min	-.01	5	-.133	1	-.032	3	-6.671e-3	1	801.776	6	1219.307	4
553	11	max	0	1	-.013	15	.08	4	5.36e-3	3	6882.5	15	4618.002	10
554		min	-.011	5	-.131	1	-.034	3	-7.404e-3	1	813.807	6	1327.128	4
555	12	max	0	1	-.013	15	.069	4	5.864e-3	3	7204.146	15	4425.177	10



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.