



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	96 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.601 k-ft
$M_z$ =	0.234 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>78%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

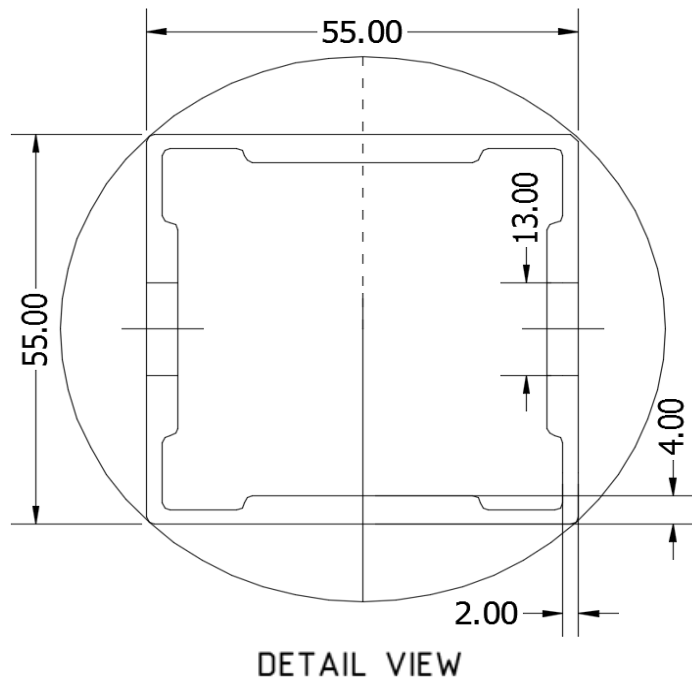
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.344 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.775 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>99%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

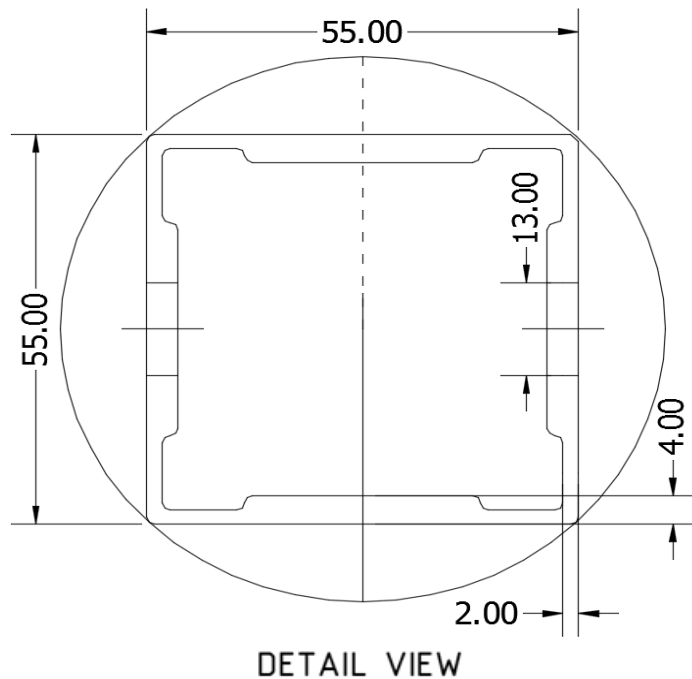
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.484 k-ft
$P_n$ =	0.590 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>37%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

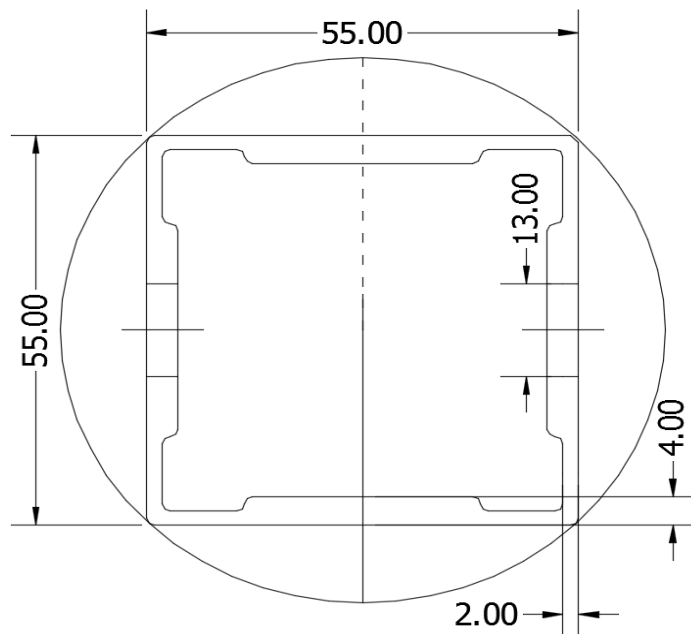
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.013 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.875 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>32%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.410 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<u>26%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

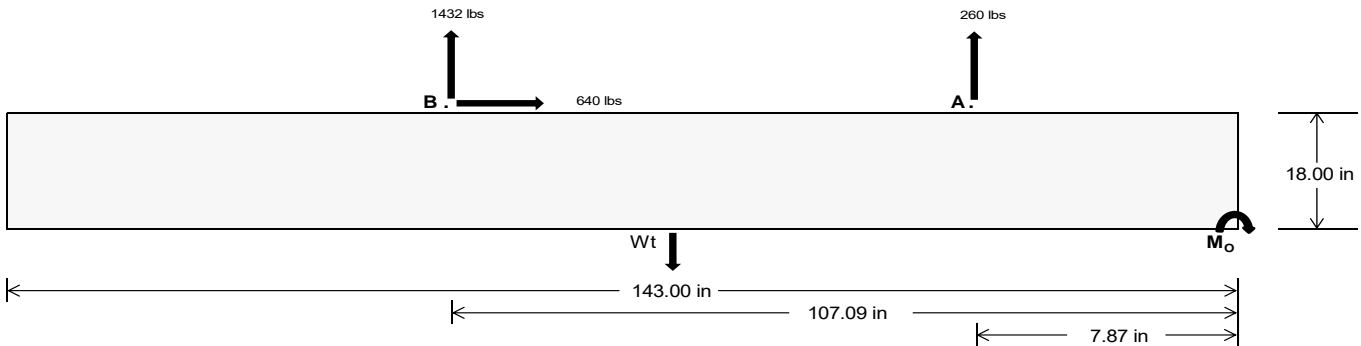
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>1091.14</u>	<u>5965.85</u> k
Compressive Load =		<u>4220.72</u>	<u>4784.73</u> k
Lateral Load =		<u>321.34</u>	<u>2662.66</u> k
Moment (Weak Axis) =		<u>0.65</u>	<u>0.35</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 166868.8$  in-lbs  
Resisting Force Required = 2333.83 lbs  
S.F. = 1.67  
Weight Required = 3889.72 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 640.10 lbs  
Friction = 0.4  
Weight Required = 1600.24 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 640.10 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1404 lbs	1404 lbs	1404 lbs	1404 lbs	1600 lbs	1600 lbs	1600 lbs	1600 lbs	2137 lbs	2137 lbs	2137 lbs	2137 lbs	-519 lbs	-519 lbs	-519 lbs	-519 lbs
$F_B$	1504 lbs	1504 lbs	1504 lbs	1504 lbs	1959 lbs	1959 lbs	1959 lbs	1959 lbs	2474 lbs	2474 lbs	2474 lbs	2474 lbs	-2863 lbs	-2863 lbs	-2863 lbs	-2863 lbs
$F_V$	142 lbs	142 lbs	142 lbs	142 lbs	1136 lbs	1136 lbs	1136 lbs	1136 lbs	948 lbs	948 lbs	948 lbs	948 lbs	-1280 lbs	-1280 lbs	-1280 lbs	-1280 lbs
$P_{total}$	10468 lbs	10684 lbs	10900 lbs	11116 lbs	11118 lbs	11334 lbs	11550 lbs	11766 lbs	12171 lbs	12387 lbs	12603 lbs	12819 lbs	1153 lbs	1283 lbs	1413 lbs	1542 lbs
$M$	3199 lbs-ft	3199 lbs-ft	3199 lbs-ft	3199 lbs-ft	4378 lbs-ft	4378 lbs-ft	4378 lbs-ft	4378 lbs-ft	5417 lbs-ft	5417 lbs-ft	5417 lbs-ft	5417 lbs-ft	3818 lbs-ft	3818 lbs-ft	3818 lbs-ft	3818 lbs-ft
$e$	0.31 ft	0.30 ft	0.29 ft	0.29 ft	0.39 ft	0.39 ft	0.38 ft	0.37 ft	0.45 ft	0.44 ft	0.43 ft	0.42 ft	3.31 ft	2.98 ft	2.70 ft	2.48 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	254.8 psf	253.8 psf	252.8 psf	251.9 psf	256.5 psf	255.4 psf	254.4 psf	253.4 psf	271.7 psf	270.2 psf	268.8 psf	267.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	347.5 psf	343.9 psf	340.5 psf	337.2 psf	383.3 psf	378.7 psf	374.3 psf	370.2 psf	428.6 psf	422.8 psf	417.2 psf	412.0 psf	99.6 psf	95.6 psf	93.8 psf	93.2 psf

Maximum Bearing Pressure = 429 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

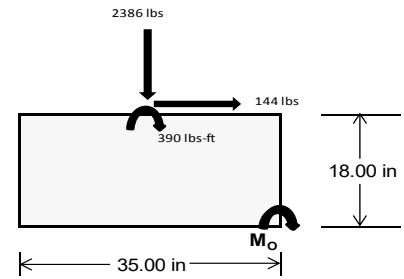
### Overturning Check

$M_o = 2874.0$  ft-lbs  
 Resisting Force Required = 1970.76 lbs  
 S.F. = 1.67  
 Weight Required = 3284.60 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	271 lbs	592 lbs	202 lbs	866 lbs	2386 lbs	812 lbs	104 lbs	173 lbs	35 lbs
$F_v$	201 lbs	197 lbs	204 lbs	149 lbs	144 lbs	158 lbs	202 lbs	198 lbs	203 lbs
$P_{total}$	9630 lbs	9951 lbs	9560 lbs	9775 lbs	11295 lbs	9721 lbs	2840 lbs	2910 lbs	2771 lbs
$M$	811 lbs-ft	799 lbs-ft	819 lbs-ft	609 lbs-ft	606 lbs-ft	641 lbs-ft	808 lbs-ft	797 lbs-ft	812 lbs-ft
$e$	0.08 ft	0.08 ft	0.09 ft	0.06 ft	0.05 ft	0.07 ft	0.28 ft	0.27 ft	0.29 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	229.1 psf	239.0 psf	226.6 psf	245.2 psf	289.1 psf	241.8 psf	33.9 psf	36.6 psf	31.7 psf
$f_{max}$	325.1 psf	333.6 psf	323.5 psf	317.3 psf	360.9 psf	317.6 psf	129.6 psf	130.9 psf	127.8 psf



Maximum Bearing Pressure = 361 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 32in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



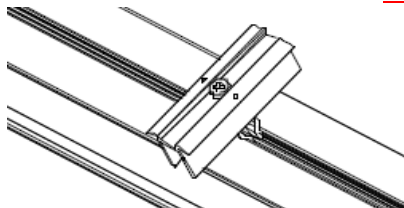
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

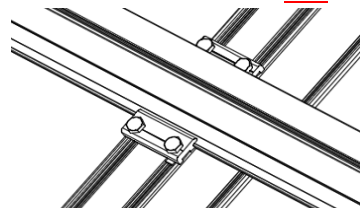
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.807 k
Allowable Uplift =	1.214 k
Utilization =	<u>66%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.296 k
Allowable Uplift =	4.357 k
Utilization =	<u>53%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.247 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

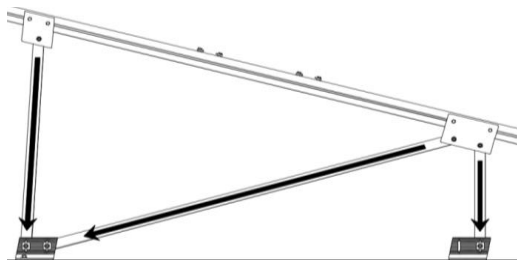
#### Rear Strut

Maximum Axial Load =	4.105 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>55%</u>

#### Diagonal Strut

Maximum Axial Load =	2.041 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>28%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

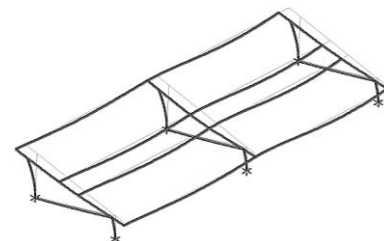
## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>0.601 ≤ 1.038, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 96 \text{ in}$$

$$J = 0.432$$

$$265.581$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 96$$

$$J = 0.432$$

$$168.894$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.41345$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77788$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 13.6277 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-65.446	-65.446	0	0
2	M14	y	-65.446	-65.446	0	0
3	M15	y	-102.844	-102.844	0	0
4	M16	y	-102.844	-102.844	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	149.592	149.592	0	0
2	M14	y	114.687	114.687	0	0
3	M15	y	62.33	62.33	0	0
4	M16	y	62.33	62.33	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	58.854	1	814.428	1	4.665	12	.004	14	.236	1	1.372	1
20			min	3.947	12	-1164.681	3	-166.734	1	-.015	2	.003	12	-1.847	3
21		11	max	58.854	1	671.563	1	-3.396	12	.015	2	.103	1	.712	1
22			min	3.947	12	-957.159	3	-131.675	1	0	15	-.002	3	-.904	3
23		12	max	58.854	1	528.698	1	-2.127	12	.015	2	.048	4	.178	1
24			min	3.947	12	-749.637	3	-96.615	1	0	15	-.005	3	-.145	3
25		13	max	58.854	1	385.834	1	-.858	12	.015	2	.023	5	.429	3
26			min	3.947	12	-542.114	3	-61.556	1	0	15	-.069	1	-.228	1
27		14	max	58.854	1	242.969	1	.749	3	.015	2	0	15	.819	3
28			min	1.66	15	-334.592	3	-30.744	4	0	15	-.108	1	-.508	1
29		15	max	58.854	1	100.105	1	8.563	1	.015	2	-.004	12	1.024	3
30			min	-7.951	5	-127.07	3	-22.979	5	0	15	-.116	1	-.66	1
31		16	max	58.854	1	80.452	3	43.623	1	.015	2	-.002	12	1.045	3
32			min	-18.44	5	-42.76	1	-21.047	5	0	15	-.093	1	-.685	1
33		17	max	58.854	1	287.975	3	78.682	1	.015	2	.002	3	.881	3
34			min	-28.93	5	-185.625	1	-19.116	5	0	15	-.068	4	-.584	1
35		18	max	58.854	1	495.497	3	113.742	1	.015	2	.047	1	.533	3
36			min	-39.419	5	-328.489	1	-17.184	5	0	15	-.075	5	-.355	1
37		19	max	58.854	1	703.019	3	148.801	1	.015	2	.164	1	0	1
38			min	-49.908	5	-471.354	1	-15.252	5	0	15	-.09	5	0	3
39	M14	1	max	47.664	4	539.398	1	-7.012	12	.013	3	.209	4	0	1
40			min	2.092	12	-573.172	3	-155.124	1	-.016	1	.013	12	0	3
41		2	max	37.175	4	396.533	1	-5.743	12	.013	3	.143	4	.439	3
42			min	2.092	12	-415.292	3	-120.065	1	-.016	1	.005	10	-.416	1
43		3	max	36.801	1	253.669	1	-4.474	12	.013	3	.085	5	.738	3
44			min	2.092	12	-257.412	3	-85.005	1	-.016	1	-.016	1	-.705	1
45		4	max	36.801	1	110.804	1	-3.205	12	.013	3	.048	5	.897	3
46			min	2.092	12	-99.533	3	-52.108	4	-.016	1	-.076	1	-.867	1
47		5	max	36.801	1	58.347	3	-.551	10	.013	3	.013	5	.915	3
48			min	-1.951	5	-32.061	1	-42.931	4	-.016	1	-.105	1	-.902	1
49		6	max	36.801	1	216.226	3	20.173	1	.013	3	-.004	12	.793	3
50			min	-12.44	5	-174.925	1	-36.89	5	-.016	1	-.102	1	-.81	1
51		7	max	36.801	1	374.106	3	55.233	1	.013	3	-.004	12	.531	3
52			min	-22.929	5	-317.79	1	-34.958	5	-.016	1	-.069	1	-.591	1
53		8	max	36.801	1	531.985	3	90.292	1	.013	3	.002	10	.128	3
54			min	-33.419	5	-460.655	1	-33.026	5	-.016	1	-.086	4	-.253	2
55		9	max	36.801	1	689.865	3	125.352	1	.013	3	.092	1	.228	1
56			min	-43.908	5	-603.519	1	-31.094	5	-.016	1	-.112	5	-.415	3
57		10	max	65.868	4	746.384	1	-4.411	12	.013	3	.219	1	.828	1
58			min	2.092	12	-847.745	3	-160.412	1	-.016	1	.002	12	-1.098	3
59		11	max	55.378	4	603.519	1	-3.141	12	.016	1	.143	4	.228	1
60			min	2.092	12	-689.865	3	-125.352	1	-.013	3	-.002	3	-.415	3
61		12	max	44.889	4	460.655	1	-1.872	12	.016	1	.083	4	.128	3
62			min	2.092	12	-531.985	3	-90.292	1	-.013	3	-.005	3	-.253	2
63		13	max	36.801	1	317.79	1	-.603	12	.016	1	.045	5	.531	3
64			min	2.092	12	-374.106	3	-55.233	1	-.013	3	-.069	1	-.591	1
65		14	max	36.801	1	174.925	1	1.133	3	.016	1	.01	5	.793	3
66			min	2.092	12	-216.226	3	-43.899	4	-.013	3	-.102	1	-.81	1
67		15	max	36.801	1	32.061	1	14.886	1	.016	1	-.003	12	.915	3
68			min	2.092	12	-58.347	3	-37.095	5	-.013	3	-.105	1	-.902	1
69		16	max	36.801	1	99.533	3	49.946	1	.016	1	0	12	.897	3
70			min	-4.853	5	-110.804	1	-35.163	5	-.013	3	-.076	1	-.867	1
71		17	max	36.801	1	257.412	3	85.005	1	.016	1	.004	3	.738	3
72			min	-15.342	5	-253.669	1	-33.232	5	-.013	3	-.092	4	-.705	1
73		18	max	36.801	1	415.292	3	120.065	1	.016	1	.075	1	.439	3
74			min	-25.831	5	-396.533	1	-31.3	5	-.013	3	-.115	5	-.416	1
75		19	max	36.801	1	573.172	3	155.124	1	.016	1	.198	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-36.321	5	-539.398	1	-29.368	5	-.013	3	-.142	5	0	3
77	M15	1	max	80.563	5	678.818	2	-6.895	12	.016	2	.274	4	0	2
78			min	-38.831	1	-322.724	3	-155.111	1	-.011	3	.012	12	0	3
79		2	max	70.074	5	494.63	2	-5.626	12	.016	2	.194	4	.25	3
80			min	-38.831	1	-239.308	3	-120.052	1	-.011	3	.005	10	-.522	2
81		3	max	59.585	5	310.442	2	-4.357	12	.016	2	.122	5	.425	3
82			min	-38.831	1	-155.891	3	-84.992	1	-.011	3	-.016	1	-.879	2
83		4	max	49.095	5	126.254	2	-3.088	12	.016	2	.071	5	.527	3
84			min	-38.831	1	-72.475	3	-67.961	4	-.011	3	-.076	1	-1.073	2
85		5	max	38.606	5	10.941	3	-.607	10	.016	2	.021	5	.554	3
86			min	-38.831	1	-57.934	2	-58.784	4	-.011	3	-.105	1	-1.104	2
87		6	max	28.117	5	94.357	3	20.186	1	.016	2	-.004	12	.507	3
88			min	-38.831	1	-242.122	2	-52.725	5	-.011	3	-.102	1	-.97	2
89		7	max	17.627	5	177.774	3	55.246	1	.016	2	-.004	12	.387	3
90			min	-38.831	1	-426.31	2	-50.793	5	-.011	3	-.088	4	-.673	2
91		8	max	7.138	5	261.19	3	90.305	1	.016	2	.002	10	.191	3
92			min	-38.831	1	-610.499	2	-48.861	5	-.011	3	-.12	4	-.224	1
93		9	max	-2.189	15	344.606	3	125.365	1	.016	2	.092	1	.412	2
94			min	-38.831	1	-794.687	2	-46.929	5	-.011	3	-.159	5	-.078	3
95		10	max	-2.662	12	978.875	2	6.669	3	.016	2	.272	4	1.2	2
96			min	-38.831	1	-266.908	12	-160.424	1	-.011	3	.003	12	-.421	3
97		11	max	-.827	15	794.687	2	-3.259	12	.011	3	.19	4	.412	2
98			min	-38.831	1	-344.606	3	-125.365	1	-.016	2	-.001	3	-.078	3
99		12	max	-2.662	12	610.499	2	-1.989	12	.011	3	.117	4	.191	3
100			min	-38.831	1	-261.19	3	-90.305	1	-.016	2	-.005	3	-.224	1
101		13	max	-2.662	12	426.31	2	-.72	12	.011	3	.065	5	.387	3
102			min	-38.831	1	-177.774	3	-68.956	4	-.016	2	-.069	1	-.673	2
103		14	max	-2.662	12	242.122	2	.946	3	.011	3	.015	5	.507	3
104			min	-41.701	4	-94.357	3	-59.778	4	-.016	2	-.102	1	-.97	2
105		15	max	-2.662	12	57.934	2	14.873	1	.011	3	-.003	12	.554	3
106			min	-52.19	4	-10.941	3	-52.93	5	-.016	2	-.105	1	-1.104	2
107		16	max	-2.662	12	72.475	3	49.933	1	.011	3	-.001	12	.527	3
108			min	-62.679	4	-126.254	2	-50.998	5	-.016	2	-.096	4	-1.073	2
109		17	max	-2.662	12	155.891	3	84.992	1	.011	3	.004	3	.425	3
110			min	-73.169	4	-310.442	2	-49.066	5	-.016	2	-.128	4	-.879	2
111		18	max	-2.662	12	239.308	3	120.052	1	.011	3	.075	1	.25	3
112			min	-83.658	4	-494.63	2	-47.134	5	-.016	2	-.166	5	-.522	2
113		19	max	-2.662	12	322.724	3	155.111	1	.011	3	.197	1	0	2
114			min	-94.147	4	-678.818	2	-45.203	5	-.016	2	-.207	5	0	5
115	M16	1	max	76.165	5	613.433	2	-6.389	12	.011	1	.192	4	0	2
116			min	-65.556	1	-271.422	3	-149.326	1	-.012	3	.01	12	0	3
117		2	max	65.676	5	429.245	2	-5.12	12	.011	1	.129	4	.204	3
118			min	-65.556	1	-188.005	3	-114.267	1	-.012	3	.003	10	-.463	2
119		3	max	55.186	5	245.057	2	-3.851	12	.011	1	.081	5	.334	3
120			min	-65.556	1	-104.589	3	-79.207	1	-.012	3	-.037	1	-.763	2
121		4	max	44.697	5	60.869	2	-2.581	12	.011	1	.048	5	.39	3
122			min	-65.556	1	-21.173	3	-46.917	4	-.012	3	-.092	1	-.899	2
123		5	max	34.208	5	62.243	3	-.21	10	.011	1	.016	5	.372	3
124			min	-65.556	1	-123.319	2	-37.74	4	-.012	3	-.115	1	-.871	2
125		6	max	23.718	5	145.66	3	25.972	1	.011	1	-.005	12	.279	3
126			min	-65.556	1	-307.507	2	-33.221	5	-.012	3	-.108	1	-.68	2
127		7	max	13.229	5	229.076	3	61.031	1	.011	1	-.004	12	.113	3
128			min	-65.556	1	-491.695	2	-31.29	5	-.012	3	-.069	1	-.325	2
129		8	max	2.74	5	312.492	3	96.091	1	.011	1	.003	2	.194	2
130			min	-65.556	1	-675.883	2	-29.358	5	-.012	3	-.072	4	-.128	3
131		9	max	-3.882	12	395.909	3	131.15	1	.011	1	.102	1	.877	2
132			min	-65.556	1	-860.072	2	-27.426	5	-.012	3	-.096	5	-.443	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-3.882	12	1044.26	2	-5.034	12	.011	1	.234	1	1.723	2
134		min	-65.556	1	-479.325	3	-166.21	1	-.012	3	.004	12	-.832	3
135	11	max	.578	5	860.072	2	-3.765	12	.012	3	.129	4	.877	2
136		min	-65.556	1	-395.909	3	-131.15	1	-.011	1	0	3	-.443	3
137	12	max	-3.882	12	675.883	2	-2.495	12	.012	3	.072	4	.194	2
138		min	-65.556	1	-312.492	3	-96.091	1	-.011	1	-.004	3	-.128	3
139	13	max	-3.882	12	491.695	2	-1.226	12	.012	3	.036	5	.113	3
140		min	-65.556	1	-229.076	3	-61.031	1	-.011	1	-.069	1	-.325	2
141	14	max	-3.882	12	307.507	2	.158	3	.012	3	.003	5	.279	3
142		min	-65.556	1	-145.66	3	-41.862	4	-.011	1	-.108	1	-.68	2
143	15	max	-3.882	12	123.319	2	9.088	1	.012	3	-.004	12	.372	3
144		min	-65.556	1	-62.243	3	-34.103	5	-.011	1	-.115	1	-.871	2
145	16	max	-3.882	12	21.173	3	44.147	1	.012	3	-.002	12	.39	3
146		min	-66.384	4	-60.869	2	-32.171	5	-.011	1	-.092	1	-.899	2
147	17	max	-3.882	12	104.589	3	79.207	1	.012	3	.001	3	.334	3
148		min	-76.873	4	-245.057	2	-30.239	5	-.011	1	-.094	4	-.763	2
149	18	max	-3.882	12	188.005	3	114.267	1	.012	3	.049	1	.204	3
150		min	-87.363	4	-429.245	2	-28.307	5	-.011	1	-.111	5	-.463	2
151	19	max	-3.882	12	271.422	3	149.326	1	.012	3	.166	1	0	2
152		min	-97.852	4	-613.433	2	-26.376	5	-.011	1	-.136	5	0	5
153	M2	1	max	1108.012	1	2.214	4	.693	1	0	0	3	0	1
154		min	-1303.178	3	.545	15	-50.432	4	0	1	0	1	0	1
155	2	max	1108.428	1	2.205	4	.693	1	0	3	0	1	0	15
156		min	-1302.866	3	.543	15	-50.792	4	0	1	-.014	4	0	4
157	3	max	1108.843	1	2.197	4	.693	1	0	3	0	1	0	15
158		min	-1302.554	3	.541	15	-51.153	4	0	1	-.028	4	-.001	4
159	4	max	1109.259	1	2.188	4	.693	1	0	3	0	1	0	15
160		min	-1302.242	3	.538	15	-51.513	4	0	1	-.043	4	-.002	4
161	5	max	1109.675	1	2.179	4	.693	1	0	3	0	1	0	15
162		min	-1301.93	3	.536	15	-51.874	4	0	1	-.057	4	-.002	4
163	6	max	1110.091	1	2.171	4	.693	1	0	3	0	1	0	15
164		min	-1301.618	3	.534	15	-52.234	4	0	1	-.072	4	-.003	4
165	7	max	1110.507	1	2.162	4	.693	1	0	3	.001	1	0	15
166		min	-1301.306	3	.532	15	-52.595	4	0	1	-.087	4	-.004	4
167	8	max	1110.923	1	2.153	4	.693	1	0	3	.001	1	-.001	15
168		min	-1300.994	3	.53	15	-52.955	4	0	1	-.101	4	-.004	4
169	9	max	1111.339	1	2.144	4	.693	1	0	3	.002	1	-.001	15
170		min	-1300.682	3	.528	15	-53.316	4	0	1	-.116	4	-.005	4
171	10	max	1111.755	1	2.136	4	.693	1	0	3	.002	1	-.001	15
172		min	-1300.371	3	.526	15	-53.676	4	0	1	-.131	4	-.005	4
173	11	max	1112.17	1	2.127	4	.693	1	0	3	.002	1	-.001	15
174		min	-1300.059	3	.524	15	-54.037	4	0	1	-.146	4	-.006	4
175	12	max	1112.586	1	2.118	4	.693	1	0	3	.002	1	-.002	15
176		min	-1299.747	3	.522	15	-54.397	4	0	1	-.162	4	-.007	4
177	13	max	1113.002	1	2.11	4	.693	1	0	3	.002	1	-.002	15
178		min	-1299.435	3	.52	15	-54.758	4	0	1	-.177	4	-.007	4
179	14	max	1113.418	1	2.101	4	.693	1	0	3	.003	1	-.002	15
180		min	-1299.123	3	.518	15	-55.118	4	0	1	-.192	4	-.008	4
181	15	max	1113.834	1	2.092	4	.693	1	0	3	.003	1	-.002	15
182		min	-1298.811	3	.516	15	-55.479	4	0	1	-.208	4	-.008	4
183	16	max	1114.25	1	2.083	4	.693	1	0	3	.003	1	-.002	15
184		min	-1298.499	3	.514	15	-55.839	4	0	1	-.223	4	-.009	4
185	17	max	1114.666	1	2.075	4	.693	1	0	3	.003	1	-.002	15
186		min	-1298.187	3	.512	15	-56.2	4	0	1	-.239	4	-.01	4
187	18	max	1115.082	1	2.066	4	.693	1	0	3	.003	1	-.003	15
188		min	-1297.875	3	.51	15	-56.56	4	0	1	-.255	4	-.01	4
189	19	max	1115.497	1	2.057	4	.693	1	0	3	.003	1	-.003	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1297.563	3	.508	15	-56.92	4	0	1	-.271	4	-.011	4
191	M3	1	max	538.447	2	9.135	4	.167	1	0	0	1	.011	4
192		min	-676.527	3	2.162	15	-3.112	5	0	4	-.005	4	.003	15
193		2	max	538.277	2	8.26	4	.167	1	0	0	1	.007	4
194		min	-676.655	3	1.956	15	-2.504	5	0	4	-.006	4	.001	12
195		3	max	538.106	2	7.386	4	.167	1	0	0	1	.003	2
196		min	-676.782	3	1.75	15	-1.895	5	0	4	-.007	4	0	3
197		4	max	537.936	2	6.511	4	.167	1	0	0	1	0	2
198		min	-676.91	3	1.545	15	-1.286	5	0	4	-.008	5	-.002	3
199		5	max	537.766	2	5.637	4	.167	1	0	0	1	0	15
200		min	-677.038	3	1.339	15	-.677	5	0	4	-.008	5	-.003	3
201		6	max	537.595	2	4.763	4	.167	1	0	0	1	-.001	15
202		min	-677.166	3	1.134	15	-.069	5	0	4	-.008	5	-.006	6
203		7	max	537.425	2	3.888	4	.598	4	0	0	1	-.002	15
204		min	-677.293	3	.928	15	.01	12	0	4	-.008	5	-.008	6
205		8	max	537.255	2	3.014	4	1.207	4	0	0	1	-.002	15
206		min	-677.421	3	.723	15	.01	12	0	4	-.008	5	-.009	6
207		9	max	537.084	2	2.139	4	1.815	4	0	0	1	-.002	15
208		min	-677.549	3	.517	15	.01	12	0	4	-.007	5	-.011	6
209		10	max	536.914	2	1.265	4	2.424	4	0	0	1	-.003	15
210		min	-677.677	3	.312	15	.01	12	0	4	-.006	5	-.011	6
211		11	max	536.744	2	.439	2	3.033	4	0	0	1	-.003	15
212		min	-677.804	3	-.015	3	.01	12	0	4	-.005	5	-.012	6
213		12	max	536.573	2	-.099	15	3.642	4	0	0	1	-.003	15
214		min	-677.932	3	-.526	3	.01	12	0	4	-.003	5	-.012	6
215		13	max	536.403	2	-.305	15	4.25	4	0	0	1	-.003	15
216		min	-678.06	3	-1.36	6	.01	12	0	4	-.001	5	-.011	6
217		14	max	536.233	2	-.511	15	4.859	4	0	0	1	-.002	15
218		min	-678.188	3	-2.234	6	.01	12	0	4	0	12	-.01	6
219		15	max	536.062	2	-.716	15	5.468	4	0	0	4	-.002	15
220		min	-678.315	3	-3.109	6	.01	12	0	4	0	12	-.009	6
221		16	max	535.892	2	-.922	15	6.076	4	0	0	4	-.002	15
222		min	-678.443	3	-3.983	6	.01	12	0	4	0	12	-.008	6
223		17	max	535.722	2	-1.127	15	6.685	4	0	0	4	-.001	15
224		min	-678.571	3	-4.858	6	.01	12	0	4	0	12	-.005	6
225		18	max	535.551	2	-1.333	15	7.294	4	0	0	4	0	15
226		min	-678.699	3	-5.732	6	.01	12	0	4	0	12	-.003	6
227		19	max	535.381	2	-1.538	15	7.903	4	0	0	4	0	1
228		min	-678.826	3	-6.607	6	.01	12	0	4	0	12	0	1
229	M4	1	max	1164.896	1	0	1	-.563	12	0	.01	4	0	1
230		min	-249.12	3	0	1	-245.662	4	0	1	0	12	0	1
231		2	max	1165.066	1	0	1	-.563	12	0	0	12	0	1
232		min	-248.993	3	0	1	-245.81	4	0	1	-.019	4	0	1
233		3	max	1165.236	1	0	1	-.563	12	0	0	12	0	1
234		min	-248.865	3	0	1	-245.957	4	0	1	-.047	4	0	1
235		4	max	1165.407	1	0	1	-.563	12	0	0	12	0	1
236		min	-248.737	3	0	1	-246.105	4	0	1	-.075	4	0	1
237		5	max	1165.577	1	0	1	-.563	12	0	0	12	0	1
238		min	-248.609	3	0	1	-246.253	4	0	1	-.103	4	0	1
239		6	max	1165.747	1	0	1	-.563	12	0	0	12	0	1
240		min	-248.482	3	0	1	-246.4	4	0	1	-.132	4	0	1
241		7	max	1165.918	1	0	1	-.563	12	0	0	12	0	1
242		min	-248.354	3	0	1	-246.548	4	0	1	-.16	4	0	1
243		8	max	1166.088	1	0	1	-.563	12	0	0	12	0	1
244		min	-248.226	3	0	1	-246.696	4	0	1	-.188	4	0	1
245		9	max	1166.258	1	0	1	-.563	12	0	0	12	0	1
246		min	-248.098	3	0	1	-246.843	4	0	1	-.217	4	0	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1166.429	1	0	1	-563	12	0	1	0	12	0	1
248			min	-247.971	3	0	1	-246.991	4	0	1	-.245	4	0	1
249		11	max	1166.599	1	0	1	-563	12	0	1	0	12	0	1
250			min	-247.843	3	0	1	-247.138	4	0	1	-.273	4	0	1
251		12	max	1166.769	1	0	1	-563	12	0	1	0	12	0	1
252			min	-247.715	3	0	1	-247.286	4	0	1	-.302	4	0	1
253		13	max	1166.94	1	0	1	-563	12	0	1	0	12	0	1
254			min	-247.587	3	0	1	-247.434	4	0	1	-.33	4	0	1
255		14	max	1167.11	1	0	1	-563	12	0	1	0	12	0	1
256			min	-247.459	3	0	1	-247.581	4	0	1	-.359	4	0	1
257		15	max	1167.28	1	0	1	-563	12	0	1	0	12	0	1
258			min	-247.332	3	0	1	-247.729	4	0	1	-.387	4	0	1
259		16	max	1167.451	1	0	1	-563	12	0	1	0	12	0	1
260			min	-247.204	3	0	1	-247.877	4	0	1	-.415	4	0	1
261		17	max	1167.621	1	0	1	-.563	12	0	1	0	12	0	1
262			min	-247.076	3	0	1	-248.024	4	0	1	-.444	4	0	1
263		18	max	1167.791	1	0	1	-563	12	0	1	-.001	12	0	1
264			min	-246.948	3	0	1	-248.172	4	0	1	-.472	4	0	1
265		19	max	1167.962	1	0	1	-563	12	0	1	-.001	12	0	1
266			min	-246.821	3	0	1	-248.32	4	0	1	-.501	4	0	1
267	M6	1	max	3402.266	1	2.602	2	0	1	0	1	0	4	0	1
268			min	-4105.44	3	.112	3	-50.959	4	0	1	0	1	0	1
269		2	max	3402.682	1	2.595	2	0	1	0	1	0	1	0	3
270			min	-4105.128	3	.107	3	-51.319	4	0	1	-.014	4	0	2
271		3	max	3403.098	1	2.588	2	0	1	0	1	0	1	0	3
272			min	-4104.816	3	.102	3	-51.679	4	0	1	-.029	4	-.001	2
273		4	max	3403.514	1	2.581	2	0	1	0	1	0	1	0	3
274			min	-4104.505	3	.097	3	-52.04	4	0	1	-.043	4	-.002	2
275		5	max	3403.93	1	2.575	2	0	1	0	1	0	1	0	3
276			min	-4104.193	3	.091	3	-52.4	4	0	1	-.058	4	-.003	2
277		6	max	3404.346	1	2.568	2	0	1	0	1	0	1	0	3
278			min	-4103.881	3	.086	3	-52.761	4	0	1	-.073	4	-.004	2
279		7	max	3404.761	1	2.561	2	0	1	0	1	0	1	0	3
280			min	-4103.569	3	.081	3	-53.121	4	0	1	-.088	4	-.004	2
281		8	max	3405.177	1	2.554	2	0	1	0	1	0	1	0	3
282			min	-4103.257	3	.076	3	-53.482	4	0	1	-.102	4	-.005	2
283		9	max	3405.593	1	2.547	2	0	1	0	1	0	1	0	3
284			min	-4102.945	3	.071	3	-53.842	4	0	1	-.118	4	-.006	2
285		10	max	3406.009	1	2.541	2	0	1	0	1	0	1	0	3
286			min	-4102.633	3	.066	3	-54.203	4	0	1	-.133	4	-.006	2
287		11	max	3406.425	1	2.534	2	0	1	0	1	0	1	0	3
288			min	-4102.321	3	.061	3	-54.563	4	0	1	-.148	4	-.007	2
289		12	max	3406.841	1	2.527	2	0	1	0	1	0	1	0	3
290			min	-4102.009	3	.056	3	-54.924	4	0	1	-.163	4	-.008	2
291		13	max	3407.257	1	2.52	2	0	1	0	1	0	1	0	3
292			min	-4101.697	3	.051	3	-55.284	4	0	1	-.179	4	-.009	2
293		14	max	3407.673	1	2.514	2	0	1	0	1	0	1	0	3
294			min	-4101.385	3	.046	3	-55.645	4	0	1	-.194	4	-.009	2
295		15	max	3408.088	1	2.507	2	0	1	0	1	0	1	0	3
296			min	-4101.074	3	.041	3	-56.005	4	0	1	-.21	4	-.01	2
297		16	max	3408.504	1	2.5	2	0	1	0	1	0	1	0	3
298			min	-4100.762	3	.035	3	-56.366	4	0	1	-.226	4	-.011	2
299		17	max	3408.92	1	2.493	2	0	1	0	1	0	1	0	3
300			min	-4100.45	3	.03	3	-56.726	4	0	1	-.242	4	-.011	2
301		18	max	3409.336	1	2.486	2	0	1	0	1	0	1	0	3
302			min	-4100.138	3	.025	3	-57.087	4	0	1	-.257	4	-.012	2
303		19	max	3409.752	1	2.48	2	0	1	0	1	0	1	0	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-4099.826	3	.02	3	-57.447	4	0	1	-.274	4	-.013	2
305	M7	1	max	1874.908	2	9.136	6	0	1	0	0	1	.013	2
306		min	-2038.599	3	2.144	15	-3.315	5	0	4	-.005	4	0	3
307		2	max	1874.737	2	8.261	6	0	1	0	0	1	.009	2
308		min	-2038.727	3	1.939	15	-2.706	5	0	4	-.006	4	-.002	3
309		3	max	1874.567	2	7.387	6	0	1	0	0	1	.006	2
310		min	-2038.855	3	1.733	15	-2.097	5	0	4	-.007	4	-.004	3
311		4	max	1874.397	2	6.512	6	0	1	0	0	1	.003	2
312		min	-2038.983	3	1.527	15	-1.489	5	0	4	-.008	4	-.005	3
313		5	max	1874.226	2	5.638	6	0	1	0	0	1	0	2
314		min	-2039.11	3	1.322	15	-.88	5	0	4	-.009	4	-.007	3
315		6	max	1874.056	2	4.764	6	0	1	0	0	1	-.001	2
316		min	-2039.238	3	1.116	15	-.271	5	0	4	-.009	4	-.008	3
317		7	max	1873.886	2	3.889	6	.355	4	0	0	1	-.002	15
318		min	-2039.366	3	.911	15	0	1	0	4	-.009	4	-.008	3
319		8	max	1873.715	2	3.015	6	.964	4	0	0	1	-.002	15
320		min	-2039.494	3	.705	15	0	1	0	4	-.008	4	-.009	4
321		9	max	1873.545	2	2.186	2	1.572	4	0	0	1	-.003	15
322		min	-2039.621	3	.395	12	0	1	0	4	-.008	4	-.011	4
323		10	max	1873.375	2	1.505	2	2.181	4	0	0	1	-.003	15
324		min	-2039.749	3	.039	3	0	1	0	4	-.007	4	-.011	4
325		11	max	1873.204	2	.823	2	2.79	4	0	0	1	-.003	15
326		min	-2039.877	3	-.472	3	0	1	0	4	-.006	4	-.012	4
327		12	max	1873.034	2	.142	2	3.398	4	0	0	1	-.003	15
328		min	-2040.005	3	-.983	3	0	1	0	4	-.004	5	-.012	4
329		13	max	1872.864	2	-.323	15	4.007	4	0	0	1	-.003	15
330		min	-2040.132	3	-1.494	3	0	1	0	4	-.003	5	-.011	4
331		14	max	1872.693	2	-.528	15	4.616	4	0	0	1	-.002	15
332		min	-2040.26	3	-2.232	4	0	1	0	4	0	5	-.01	4
333		15	max	1872.523	2	-.734	15	5.224	4	0	.002	4	-.002	15
334		min	-2040.388	3	-3.106	4	0	1	0	4	0	1	-.009	4
335		16	max	1872.353	2	-.939	15	5.833	4	0	.004	4	-.002	15
336		min	-2040.516	3	-3.981	4	0	1	0	4	0	1	-.008	4
337		17	max	1872.182	2	-1.145	15	6.442	4	0	.007	4	-.001	15
338		min	-2040.644	3	-4.855	4	0	1	0	4	0	1	-.005	4
339		18	max	1872.012	2	-1.35	15	7.051	4	0	.011	4	0	15
340		min	-2040.771	3	-5.73	4	0	1	0	4	0	1	-.003	4
341		19	max	1871.841	2	-1.556	15	7.659	4	0	.014	4	0	1
342		min	-2040.899	3	-6.604	4	0	1	0	4	0	1	0	1
343	M8	1	max	3243.64	1	0	1	0	1	0	.008	4	0	1
344		min	-841.636	3	0	1	-237.074	4	0	1	0	1	0	1
345		2	max	3243.81	1	0	1	0	1	0	0	1	0	1
346		min	-841.508	3	0	1	-237.222	4	0	1	-.019	4	0	1
347		3	max	3243.981	1	0	1	0	1	0	0	1	0	1
348		min	-841.38	3	0	1	-237.37	4	0	1	-.046	4	0	1
349		4	max	3244.151	1	0	1	0	1	0	0	1	0	1
350		min	-841.253	3	0	1	-237.517	4	0	1	-.073	4	0	1
351		5	max	3244.321	1	0	1	0	1	0	0	1	0	1
352		min	-841.125	3	0	1	-237.665	4	0	1	-.101	4	0	1
353		6	max	3244.492	1	0	1	0	1	0	0	1	0	1
354		min	-840.997	3	0	1	-237.812	4	0	1	-.128	4	0	1
355		7	max	3244.662	1	0	1	0	1	0	0	1	0	1
356		min	-840.869	3	0	1	-237.96	4	0	1	-.155	4	0	1
357		8	max	3244.832	1	0	1	0	1	0	0	1	0	1
358		min	-840.742	3	0	1	-238.108	4	0	1	-.183	4	0	1
359		9	max	3245.003	1	0	1	0	1	0	0	1	0	1
360		min	-840.614	3	0	1	-238.255	4	0	1	-.21	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3245.173	1	0	1	0	1	0	1	0	1	0	1
362			min	-840.486	3	0	1	-238.403	4	0	1	-.237	4	0	1
363		11	max	3245.343	1	0	1	0	1	0	1	0	1	0	1
364			min	-840.358	3	0	1	-238.551	4	0	1	-.265	4	0	1
365		12	max	3245.514	1	0	1	0	1	0	1	0	1	0	1
366			min	-840.23	3	0	1	-238.698	4	0	1	-.292	4	0	1
367		13	max	3245.684	1	0	1	0	1	0	1	0	1	0	1
368			min	-840.103	3	0	1	-238.846	4	0	1	-.32	4	0	1
369		14	max	3245.854	1	0	1	0	1	0	1	0	1	0	1
370			min	-839.975	3	0	1	-238.994	4	0	1	-.347	4	0	1
371		15	max	3246.025	1	0	1	0	1	0	1	0	1	0	1
372			min	-839.847	3	0	1	-239.141	4	0	1	-.374	4	0	1
373		16	max	3246.195	1	0	1	0	1	0	1	0	1	0	1
374			min	-839.719	3	0	1	-239.289	4	0	1	-.402	4	0	1
375		17	max	3246.365	1	0	1	0	1	0	1	0	1	0	1
376			min	-839.592	3	0	1	-239.436	4	0	1	-.429	4	0	1
377		18	max	3246.536	1	0	1	0	1	0	1	0	1	0	1
378			min	-839.464	3	0	1	-239.584	4	0	1	-.457	4	0	1
379		19	max	3246.706	1	0	1	0	1	0	1	0	1	0	1
380			min	-839.336	3	0	1	-239.732	4	0	1	-.484	4	0	1
381	M10	1	max	1108.012	1	2.102	6	-.045	12	0	1	0	4	0	1
382			min	-1303.178	3	.47	15	-50.801	4	0	3	0	3	0	1
383		2	max	1108.428	1	2.094	6	-.045	12	0	1	0	10	0	15
384			min	-1302.866	3	.468	15	-51.161	4	0	3	-.014	4	0	6
385		3	max	1108.843	1	2.085	6	-.045	12	0	1	0	10	0	15
386			min	-1302.554	3	.465	15	-51.522	4	0	3	-.029	4	-.001	6
387		4	max	1109.259	1	2.076	6	-.045	12	0	1	0	10	0	15
388			min	-1302.242	3	.463	15	-51.882	4	0	3	-.043	4	-.002	6
389		5	max	1109.675	1	2.068	6	-.045	12	0	1	0	10	0	15
390			min	-1301.93	3	.461	15	-52.243	4	0	3	-.058	4	-.002	6
391		6	max	1110.091	1	2.059	6	-.045	12	0	1	0	12	0	15
392			min	-1301.618	3	.459	15	-52.603	4	0	3	-.072	4	-.003	6
393		7	max	1110.507	1	2.05	6	-.045	12	0	1	0	12	0	15
394			min	-1301.306	3	.457	15	-52.963	4	0	3	-.087	4	-.003	6
395		8	max	1110.923	1	2.041	6	-.045	12	0	1	0	12	0	15
396			min	-1300.994	3	.455	15	-53.324	4	0	3	-.102	4	-.004	6
397		9	max	1111.339	1	2.033	6	-.045	12	0	1	0	12	-.001	15
398			min	-1300.682	3	.453	15	-53.684	4	0	3	-.117	4	-.005	6
399		10	max	1111.755	1	2.024	6	-.045	12	0	1	0	12	-.001	15
400			min	-1300.371	3	.451	15	-54.045	4	0	3	-.132	4	-.005	6
401		11	max	1112.17	1	2.015	6	-.045	12	0	1	0	12	-.001	15
402			min	-1300.059	3	.449	15	-54.405	4	0	3	-.147	4	-.006	6
403		12	max	1112.586	1	2.007	6	-.045	12	0	1	0	12	-.001	15
404			min	-1299.747	3	.447	15	-54.766	4	0	3	-.163	4	-.006	6
405		13	max	1113.002	1	1.998	6	-.045	12	0	1	0	12	-.002	15
406			min	-1299.435	3	.445	15	-55.126	4	0	3	-.178	4	-.007	6
407		14	max	1113.418	1	1.989	6	-.045	12	0	1	0	12	-.002	15
408			min	-1299.123	3	.443	15	-55.487	4	0	3	-.194	4	-.007	6
409		15	max	1113.834	1	1.98	6	-.045	12	0	1	0	12	-.002	15
410			min	-1298.811	3	.441	15	-55.847	4	0	3	-.209	4	-.008	6
411		16	max	1114.25	1	1.972	6	-.045	12	0	1	0	12	-.002	15
412			min	-1298.499	3	.439	15	-56.208	4	0	3	-.225	4	-.009	6
413		17	max	1114.666	1	1.963	6	-.045	12	0	1	0	12	-.002	15
414			min	-1298.187	3	.437	15	-56.568	4	0	3	-.241	4	-.009	6
415		18	max	1115.082	1	1.954	6	-.045	12	0	1	0	12	-.002	15
416			min	-1297.875	3	.435	15	-56.929	4	0	3	-.257	4	-.01	6
417		19	max	1115.497	1	1.946	6	-.045	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1297.563	3	.433	15	-57.289	4	0	3	-.273	4	-.01	6
419	M11	1	max	538.447	2	9.069	6	-.01	12	0	1	0	12	.01	6
420			min	-676.527	3	2.117	15	-3.142	4	0	4	-.005	4	.002	15
421		2	max	538.277	2	8.194	6	-.01	12	0	1	0	12	.006	2
422			min	-676.655	3	1.912	15	-2.534	4	0	4	-.006	4	.001	15
423		3	max	538.106	2	7.32	6	-.01	12	0	1	0	12	.003	2
424			min	-676.782	3	1.706	15	-1.925	4	0	4	-.007	4	0	3
425		4	max	537.936	2	6.445	6	-.01	12	0	1	0	12	0	2
426			min	-676.91	3	1.5	15	-1.316	4	0	4	-.008	4	-.002	3
427		5	max	537.766	2	5.571	6	-.01	12	0	1	0	12	0	15
428			min	-677.038	3	1.295	15	-.708	4	0	4	-.008	4	-.004	4
429		6	max	537.595	2	4.696	6	-.01	12	0	1	0	12	-.002	15
430			min	-677.166	3	1.089	15	-.167	1	0	4	-.008	4	-.006	4
431		7	max	537.425	2	3.822	6	.522	5	0	1	0	12	-.002	15
432			min	-677.293	3	.884	15	-.167	1	0	4	-.008	4	-.008	4
433		8	max	537.255	2	2.947	6	1.131	5	0	1	0	12	-.002	15
434			min	-677.421	3	.678	15	-.167	1	0	4	-.008	4	-.01	4
435		9	max	537.084	2	2.073	6	1.739	5	0	1	0	12	-.003	15
436			min	-677.549	3	.473	15	-.167	1	0	4	-.007	4	-.011	4
437		10	max	536.914	2	1.199	6	2.348	5	0	1	0	12	-.003	15
438			min	-677.677	3	.267	15	-.167	1	0	4	-.006	4	-.012	4
439		11	max	536.744	2	.439	2	2.957	5	0	1	0	12	-.003	15
440			min	-677.804	3	-.015	3	-.167	1	0	4	-.005	4	-.012	4
441		12	max	536.573	2	-.144	15	3.565	5	0	1	0	12	-.003	15
442			min	-677.932	3	-.552	4	-.167	1	0	4	-.004	4	-.012	4
443		13	max	536.403	2	-.349	15	4.174	5	0	1	0	12	-.003	15
444			min	-678.06	3	-1.426	4	-.167	1	0	4	-.002	4	-.012	4
445		14	max	536.233	2	-.555	15	4.783	5	0	1	0	5	-.003	15
446			min	-678.188	3	-2.3	4	-.167	1	0	4	-.001	1	-.011	4
447		15	max	536.062	2	-.761	15	5.392	5	0	1	.003	5	-.002	15
448			min	-678.315	3	-3.175	4	-.167	1	0	4	-.001	1	-.009	4
449		16	max	535.892	2	-.966	15	6	5	0	1	.006	5	-.002	15
450			min	-678.443	3	-4.049	4	-.167	1	0	4	-.001	1	-.008	4
451		17	max	535.722	2	-1.172	15	6.609	5	0	1	.009	5	-.001	15
452			min	-678.571	3	-4.924	4	-.167	1	0	4	-.001	1	-.005	4
453		18	max	535.551	2	-1.377	15	7.218	5	0	1	.012	5	0	15
454			min	-678.699	3	-5.798	4	-.167	1	0	4	-.001	1	-.003	4
455		19	max	535.381	2	-1.583	15	7.826	5	0	1	.016	5	0	1
456			min	-678.826	3	-6.673	4	-.167	1	0	4	-.001	1	0	1
457	M12	1	max	1164.896	1	0	1	9.801	1	0	1	.009	5	0	1
458			min	-249.12	3	0	1	-240.427	4	0	1	0	1	0	1
459		2	max	1165.066	1	0	1	9.801	1	0	1	0	1	0	1
460			min	-248.993	3	0	1	-240.575	4	0	1	-.018	4	0	1
461		3	max	1165.236	1	0	1	9.801	1	0	1	.001	1	0	1
462			min	-248.865	3	0	1	-240.722	4	0	1	-.046	4	0	1
463		4	max	1165.407	1	0	1	9.801	1	0	1	.003	1	0	1
464			min	-248.737	3	0	1	-240.87	4	0	1	-.074	4	0	1
465		5	max	1165.577	1	0	1	9.801	1	0	1	.004	1	0	1
466			min	-248.609	3	0	1	-241.018	4	0	1	-.101	4	0	1
467		6	max	1165.747	1	0	1	9.801	1	0	1	.005	1	0	1
468			min	-248.482	3	0	1	-241.165	4	0	1	-.129	4	0	1
469		7	max	1165.918	1	0	1	9.801	1	0	1	.006	1	0	1
470			min	-248.354	3	0	1	-241.313	4	0	1	-.157	4	0	1
471		8	max	1166.088	1	0	1	9.801	1	0	1	.007	1	0	1
472			min	-248.226	3	0	1	-241.46	4	0	1	-.185	4	0	1
473		9	max	1166.258	1	0	1	9.801	1	0	1	.008	1	0	1
474			min	-248.098	3	0	1	-241.608	4	0	1	-.212	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1166.429	1	0	1	9.801	1	0	1	.009	1	0	1
476		min	-247.971	3	0	1	-241.756	4	0	1	-.24	4	0	1
477	11	max	1166.599	1	0	1	9.801	1	0	1	.01	1	0	1
478		min	-247.843	3	0	1	-241.903	4	0	1	-.268	4	0	1
479	12	max	1166.769	1	0	1	9.801	1	0	1	.012	1	0	1
480		min	-247.715	3	0	1	-242.051	4	0	1	-.296	4	0	1
481	13	max	1166.94	1	0	1	9.801	1	0	1	.013	1	0	1
482		min	-247.587	3	0	1	-242.199	4	0	1	-.323	4	0	1
483	14	max	1167.11	1	0	1	9.801	1	0	1	.014	1	0	1
484		min	-247.459	3	0	1	-242.346	4	0	1	-.351	4	0	1
485	15	max	1167.28	1	0	1	9.801	1	0	1	.015	1	0	1
486		min	-247.332	3	0	1	-242.494	4	0	1	-.379	4	0	1
487	16	max	1167.451	1	0	1	9.801	1	0	1	.016	1	0	1
488		min	-247.204	3	0	1	-242.641	4	0	1	-.407	4	0	1
489	17	max	1167.621	1	0	1	9.801	1	0	1	.017	1	0	1
490		min	-247.076	3	0	1	-242.789	4	0	1	-.435	4	0	1
491	18	max	1167.791	1	0	1	9.801	1	0	1	.018	1	0	1
492		min	-246.948	3	0	1	-242.937	4	0	1	-.463	4	0	1
493	19	max	1167.962	1	0	1	9.801	1	0	1	.019	1	0	1
494		min	-246.821	3	0	1	-243.084	4	0	1	-.491	4	0	1
495	M1	1	max	148.806	1	702.97	3	49.874	5	0	.164	1	0	15
496		min	-15.252	5	-469.288	1	-58.773	1	0	3	-.09	5	-.015	2
497	2	max	149.382	1	701.782	3	51.334	5	0	1	.128	1	.278	1
498		min	-14.983	5	-470.871	1	-58.773	1	0	3	-.058	5	-.44	3
499	3	max	437.388	3	570.023	1	3.579	5	0	3	.091	1	.559	1
500		min	-280.795	2	-534.974	3	-58.26	1	0	1	-.026	5	-.861	3
501	4	max	437.82	3	568.44	1	5.039	5	0	3	.055	1	.206	1
502		min	-280.219	2	-536.162	3	-58.26	1	0	1	-.024	5	-.529	3
503	5	max	438.253	3	566.856	1	6.499	5	0	3	.019	1	-.005	15
504		min	-279.643	2	-537.349	3	-58.26	1	0	1	-.02	5	-.196	3
505	6	max	438.685	3	565.273	1	7.96	5	0	3	-.001	12	.138	3
506		min	-279.066	2	-538.537	3	-58.26	1	0	1	-.019	4	-.509	2
507	7	max	439.117	3	563.69	1	9.42	5	0	3	-.003	12	.473	3
508		min	-278.49	2	-539.724	3	-58.26	1	0	1	-.054	1	-.848	1
509	8	max	439.549	3	562.107	1	10.88	5	0	3	-.003	15	.808	3
510		min	-277.914	2	-540.911	3	-58.26	1	0	1	-.09	1	-1.197	1
511	9	max	450.633	3	45.104	2	48.537	5	0	9	.058	1	.942	3
512		min	-217.545	2	.475	15	-95.104	1	0	3	-.126	5	-1.362	1
513	10	max	451.065	3	43.521	2	49.997	5	0	9	0	10	.921	3
514		min	-216.968	2	-.007	5	-95.104	1	0	3	-.096	4	-1.377	2
515	11	max	451.497	3	41.937	2	51.458	5	0	9	-.004	12	.9	3
516		min	-216.392	2	-1.995	4	-95.104	1	0	3	-.076	4	-1.404	2
517	12	max	462.395	3	358.921	3	137.965	5	0	2	.088	1	.788	3
518		min	-155.951	2	-636.22	2	-56.291	1	0	3	-.217	5	-1.245	2
519	13	max	462.827	3	357.734	3	139.425	5	0	2	.053	1	.566	3
520		min	-155.375	2	-637.803	2	-56.291	1	0	3	-.131	5	-.85	1
521	14	max	463.26	3	356.547	3	140.885	5	0	2	.018	1	.344	3
522		min	-154.798	2	-639.386	2	-56.291	1	0	3	-.044	5	-.469	1
523	15	max	463.692	3	355.359	3	142.345	5	0	2	.044	5	.123	3
524		min	-154.222	2	-640.97	2	-56.291	1	0	3	-.017	1	-.088	1
525	16	max	464.124	3	354.172	3	143.805	5	0	2	.132	5	.342	2
526		min	-153.646	2	-642.553	2	-56.291	1	0	3	-.052	1	-.097	3
527	17	max	464.556	3	352.985	3	145.266	5	0	2	.222	5	.742	2
528		min	-153.07	2	-644.136	2	-56.291	1	0	3	-.087	1	-.316	3
529	18	max	26.106	5	615.747	2	-3.882	12	0	5	.188	5	.373	2
530		min	-149.898	1	-270.324	3	-99.33	4	0	2	-.126	1	-1.156	3
531	19	max	26.375	5	614.164	2	-3.882	12	0	5	.136	5	.012	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-149.322	1	-271.512	3	-97.87	4	0	2	-.166	1	-.011	1
533	M5	max	333.459	1	2329.311	3	82.542	5	0	1	0	1	.03	2
534		min	9.332	12	-1620.071	1	0	1	0	4	-.182	4	0	15
535		max	334.035	1	2328.124	3	84.002	5	0	1	0	1	1.033	1
536		min	9.62	12	-1621.655	1	0	1	0	4	-.131	4	-1.438	3
537		max	1353.395	3	1557.998	1	39.247	4	0	4	0	1	2.005	1
538		min	-908.06	2	-1584.215	3	0	1	0	1	-.079	4	-2.839	3
539		max	1353.827	3	1556.415	1	40.707	4	0	4	0	1	1.039	1
540		min	-907.484	2	-1585.402	3	0	1	0	1	-.054	4	-1.856	3
541		max	1354.26	3	1554.832	1	42.168	4	0	4	0	1	.073	1
542		min	-906.908	2	-1586.589	3	0	1	0	1	-.029	4	-.871	3
543		max	1354.692	3	1553.249	1	43.628	4	0	4	0	1	.114	3
544		min	-906.332	2	-1587.777	3	0	1	0	1	-.002	5	-.919	2
545		max	1355.124	3	1551.665	1	45.088	4	0	4	.025	4	1.099	3
546		min	-905.755	2	-1588.964	3	0	1	0	1	0	1	-1.855	1
547		max	1355.556	3	1550.082	1	46.548	4	0	4	.054	4	2.086	3
548		min	-905.179	2	-1590.151	3	0	1	0	1	0	1	-2.817	1
549		max	1368.954	3	152.402	2	160.851	4	0	1	0	1	2.406	3
550		min	-775.751	2	.478	15	0	1	0	1	-.182	4	-3.202	1
551		max	1369.387	3	150.819	2	162.312	4	0	1	0	1	2.324	3
552		min	-775.175	2	0	15	0	1	0	1	-.082	5	-3.252	1
553		max	1369.819	3	149.236	2	163.772	4	0	1	.019	4	2.242	3
554		min	-774.598	2	-1.803	6	0	1	0	1	0	1	-3.344	2
555		max	1383.589	3	1019.334	3	187.361	4	0	1	0	1	1.962	3
556		min	-645.314	2	-1787.869	2	0	1	0	4	-.304	4	-2.987	2
557		max	1384.021	3	1018.146	3	188.821	4	0	1	0	1	1.33	3
558		min	-644.738	2	-1789.452	2	0	1	0	4	-.188	4	-1.877	2
559		max	1384.453	3	1016.959	3	190.281	4	0	1	0	1	.698	3
560		min	-644.162	2	-1791.035	2	0	1	0	4	-.07	4	-.812	1
561		max	1384.885	3	1015.771	3	191.741	4	0	1	.049	4	.346	2
562		min	-643.585	2	-1792.618	2	0	1	0	4	0	1	0	15
563		max	1385.317	3	1014.584	3	193.202	4	0	1	.168	4	1.459	2
564		min	-643.009	2	-1794.201	2	0	1	0	4	0	1	-.563	3
565		max	1385.749	3	1013.397	3	194.662	4	0	1	.288	4	2.573	2
566		min	-642.433	2	-1795.784	2	0	1	0	4	0	1	-1.192	3
567		max	-10.355	12	2093.016	2	0	1	0	4	.287	4	1.316	2
568		min	-333.003	1	-957.78	3	-30.826	5	0	1	0	1	-.62	3
569		max	-10.067	12	2091.433	2	0	1	0	4	.269	4	.021	1
570		min	-332.427	1	-958.967	3	-29.366	5	0	1	0	1	-.025	3
571	M9	max	148.806	1	702.97	3	69.351	4	0	3	-.011	12	0	15
572		min	6.757	12	-469.288	1	3.947	12	0	4	-.164	1	-.015	2
573		max	149.382	1	701.782	3	70.811	4	0	3	-.009	12	.278	1
574		min	7.045	12	-470.871	1	3.947	12	0	4	-.128	1	-.44	3
575		max	437.388	3	570.023	1	58.26	1	0	1	-.006	12	.559	1
576		min	-280.795	2	-534.974	3	3.898	12	0	3	-.091	1	-.861	3
577		max	437.82	3	568.44	1	58.26	1	0	1	-.004	12	.206	1
578		min	-280.219	2	-536.162	3	3.898	12	0	3	-.055	1	-.529	3
579		max	438.253	3	566.856	1	58.26	1	0	1	-.001	12	-.005	15
580		min	-279.643	2	-537.349	3	3.898	12	0	3	-.027	4	-.196	3
581		max	438.685	3	565.273	1	58.26	1	0	1	.017	1	.138	3
582		min	-279.066	2	-538.537	3	3.898	12	0	3	-.014	5	-.509	2
583		max	439.117	3	563.69	1	58.26	1	0	1	.054	1	.473	3
584		min	-278.49	2	-539.724	3	3.898	12	0	3	-.004	5	-.848	1
585		max	439.549	3	562.107	1	58.26	1	0	1	.09	1	.808	3
586		min	-277.914	2	-540.911	3	3.898	12	0	3	.004	15	-1.197	1
587		max	450.633	3	45.104	2	95.104	1	0	3	-.004	12	.942	3
588		min	-217.545	2	.489	15	5.986	12	0	9	-.145	4	-1.362	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	451.065	3	43.521	2	95.104	1	0	3	0	1	.921	3
590		min	-216.968	2	.011	15	5.986	12	0	9	-.095	4	-1.377	2
591	11	max	451.497	3	41.937	2	95.104	1	0	3	.06	1	.9	3
592		min	-216.392	2	-1.883	6	5.986	12	0	9	-.057	5	-1.404	2
593	12	max	462.395	3	358.921	3	158.12	4	0	3	-.005	12	.788	3
594		min	-155.951	2	-636.22	2	3.328	12	0	2	-.248	4	-1.245	2
595	13	max	462.827	3	357.734	3	159.58	4	0	3	-.003	12	.566	3
596		min	-155.375	2	-637.803	2	3.328	12	0	2	-.15	4	-.85	1
597	14	max	463.26	3	356.547	3	161.04	4	0	3	-.001	12	.344	3
598		min	-154.798	2	-639.386	2	3.328	12	0	2	-.05	4	-.469	1
599	15	max	463.692	3	355.359	3	162.5	4	0	3	.05	4	.123	3
600		min	-154.222	2	-640.97	2	3.328	12	0	2	0	12	-.088	1
601	16	max	464.124	3	354.172	3	163.96	4	0	3	.151	4	.342	2
602		min	-153.646	2	-642.553	2	3.328	12	0	2	.003	12	-.097	3
603	17	max	464.556	3	352.985	3	165.42	4	0	3	.254	4	.742	2
604		min	-153.07	2	-644.136	2	3.328	12	0	2	.005	12	-.316	3
605	18	max	-6.678	12	615.747	2	65.634	1	0	2	.231	4	.373	2
606		min	-149.898	1	-270.324	3	-77.775	5	0	3	.007	12	-.156	3
607	19	max	-6.389	12	614.164	2	65.634	1	0	2	.192	4	.012	3
608		min	-149.322	1	-271.512	3	-76.315	5	0	3	.01	12	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.211	2	.009	3	1.43e-2	2	NC	1	NC	1
2			min	-.625	4	-.054	3	-.005	2	-3.449e-3	3	NC	1	NC	1
3		2	max	0	1	.13	2	.019	1	1.546e-2	2	NC	4	NC	2
4			min	-.625	4	.004	15	-.012	5	-3.148e-3	3	1096.985	3	9832.153	1
5		3	max	0	1	.263	3	.045	1	1.661e-2	2	NC	5	NC	2
6			min	-.625	4	.002	15	-.015	5	-2.848e-3	3	604.503	3	4233.635	1
7		4	max	0	1	.352	3	.067	1	1.777e-2	2	NC	5	NC	3
8			min	-.625	4	-.007	9	-.012	5	-2.547e-3	3	472.976	3	2878.751	1
9		5	max	0	1	.375	3	.077	1	1.892e-2	2	NC	5	NC	3
10			min	-.625	4	-.005	9	-.005	5	-2.247e-3	3	446.793	3	2502.836	1
11		6	max	0	1	.336	3	.072	1	2.008e-2	2	NC	5	NC	3
12			min	-.625	4	.002	15	0	10	-1.946e-3	3	491.949	3	2648.96	1
13		7	max	0	1	.246	3	.055	1	2.124e-2	2	NC	4	NC	2
14			min	-.625	4	.004	15	-.003	10	-1.646e-3	3	639	3	3490.619	1
15		8	max	0	1	.261	2	.029	1	2.239e-2	2	NC	4	NC	2
16			min	-.625	4	.006	15	-.007	10	-1.345e-3	3	1039.437	3	6572.367	1
17		9	max	0	1	.337	2	.026	3	2.355e-2	2	NC	4	NC	1
18			min	-.625	4	.008	15	-.013	2	-1.044e-3	3	1521.558	2	NC	1
19		10	max	0	1	.371	2	.026	3	2.471e-2	2	NC	5	NC	1
20			min	-.625	4	-.023	3	-.018	2	-7.439e-4	3	1201.829	2	NC	1
21		11	max	0	12	.337	2	.026	3	2.355e-2	2	NC	4	NC	1
22			min	-.625	4	.008	15	-.013	2	-1.044e-3	3	1521.558	2	NC	1
23		12	max	0	12	.261	2	.029	1	2.239e-2	2	NC	4	NC	2
24			min	-.625	4	.006	15	-.009	5	-1.345e-3	3	1039.437	3	6572.367	1
25		13	max	0	12	.246	3	.055	1	2.124e-2	2	NC	4	NC	2
26			min	-.625	4	.004	15	-.004	5	-1.646e-3	3	639	3	3490.619	1
27		14	max	0	12	.336	3	.072	1	2.008e-2	2	NC	5	NC	3
28			min	-.625	4	.002	15	0	10	-1.946e-3	3	491.949	3	2648.96	1
29		15	max	0	12	.375	3	.077	1	1.892e-2	2	NC	5	NC	3
30			min	-.625	4	-.005	9	0	10	-2.247e-3	3	446.793	3	2502.836	1
31		16	max	0	12	.352	3	.067	1	1.777e-2	2	NC	5	NC	3
32			min	-.625	4	-.007	9	0	10	-2.547e-3	3	472.976	3	2878.751	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.263	3	.045	1	1.661e-2	2	NC	5	NC	2
34		min	-.625	4	.002	15	0	10	-2.848e-3	3	604.503	3	4233.635	1
35	18	max	0	12	.13	2	.019	4	1.546e-2	2	NC	4	NC	2
36		min	-.625	4	.003	15	-.002	10	-3.148e-3	3	1096.985	3	9366.349	4
37	19	max	0	12	.211	2	.009	3	1.43e-2	2	NC	1	NC	1
38		min	-.625	4	-.054	3	-.005	2	-3.449e-3	3	NC	1	NC	1
39	M14	1	max	0	.39	3	.008	3	8.08e-3	1	NC	1	NC	1
40		min	-.479	4	-.623	2	-.004	2	-5.934e-3	3	NC	1	NC	1
41	2	max	0	1	.61	3	.012	1	9.299e-3	1	NC	5	NC	1
42		min	-.479	4	-.867	1	-.018	5	-6.944e-3	3	783.054	1	NC	1
43	3	max	0	1	.802	3	.034	1	1.052e-2	1	NC	5	NC	2
44		min	-.479	4	-1.087	1	-.023	5	-7.955e-3	3	412.93	1	5617.22	1
45	4	max	0	1	.949	3	.055	1	1.174e-2	1	NC	15	NC	2
46		min	-.479	4	-1.263	1	-.016	5	-8.965e-3	3	299.434	1	3522.416	1
47	5	max	0	1	1.041	3	.066	1	1.296e-2	1	NC	15	NC	3
48		min	-.479	4	-1.386	1	-.004	5	-9.976e-3	3	251.389	1	2926.394	1
49	6	max	0	1	1.076	3	.064	1	1.418e-2	1	NC	15	NC	3
50		min	-.479	4	-1.453	1	0	10	-1.099e-2	3	231.21	1	3006.784	1
51	7	max	0	1	1.062	3	.05	1	1.54e-2	1	9953.375	15	NC	2
52		min	-.479	4	-1.469	1	-.003	10	-1.2e-2	3	226.695	1	3875.526	1
53	8	max	0	1	1.015	3	.034	4	1.662e-2	1	NC	15	NC	2
54		min	-.479	4	-1.449	1	-.006	10	-1.301e-2	3	232.135	1	5552.334	4
55	9	max	0	1	.961	3	.023	4	1.783e-2	1	NC	15	NC	1
56		min	-.479	4	-1.415	1	-.011	2	-1.402e-2	3	242.254	1	7984.891	4
57	10	max	0	1	.934	3	.023	3	1.905e-2	1	NC	15	NC	1
58		min	-.479	4	-1.395	1	-.016	2	-1.503e-2	3	248.393	1	NC	1
59	11	max	0	12	.961	3	.023	3	1.783e-2	1	NC	15	NC	1
60		min	-.479	4	-1.415	1	-.019	5	-1.402e-2	3	242.254	1	NC	1
61	12	max	0	12	1.015	3	.027	1	1.662e-2	1	NC	15	NC	2
62		min	-.479	4	-1.449	1	-.022	5	-1.301e-2	3	232.135	1	7140.159	1
63	13	max	0	12	1.062	3	.05	1	1.54e-2	1	9953.203	15	NC	2
64		min	-.479	4	-1.469	1	-.015	5	-1.2e-2	3	226.695	1	3875.526	1
65	14	max	0	12	1.076	3	.064	1	1.418e-2	1	NC	15	NC	3
66		min	-.479	4	-1.453	1	-.002	5	-1.099e-2	3	231.21	1	3006.784	1
67	15	max	0	12	1.041	3	.066	1	1.296e-2	1	NC	15	NC	3
68		min	-.479	4	-1.386	1	0	10	-9.976e-3	3	251.389	1	2926.394	1
69	16	max	0	12	.949	3	.055	1	1.174e-2	1	NC	15	NC	2
70		min	-.479	4	-1.263	1	0	10	-8.965e-3	3	299.434	1	3522.416	1
71	17	max	0	12	.802	3	.035	4	1.052e-2	1	NC	5	NC	2
72		min	-.479	4	-1.087	1	0	10	-7.955e-3	3	412.93	1	5288.841	4
73	18	max	0	12	.61	3	.024	4	9.299e-3	1	NC	5	NC	1
74		min	-.479	4	-.867	1	-.002	10	-6.944e-3	3	783.054	1	7825.715	4
75	19	max	0	12	.39	3	.008	3	8.08e-3	1	NC	1	NC	1
76		min	-.479	4	-.623	2	-.004	2	-5.934e-3	3	NC	1	NC	1
77	M15	1	max	0	.4	3	.007	3	4.997e-3	3	NC	1	NC	1
78		min	-.394	4	-.622	2	-.004	2	-8.304e-3	2	NC	1	NC	1
79	2	max	0	12	.56	3	.013	1	5.837e-3	3	NC	5	NC	1
80		min	-.394	4	-.901	2	-.027	5	-9.562e-3	2	688.76	2	7472.185	5
81	3	max	0	12	.706	3	.035	1	6.676e-3	3	NC	5	NC	2
82		min	-.394	4	-1.148	2	-.034	5	-1.082e-2	2	365.242	2	5581.144	1
83	4	max	0	12	.825	3	.055	1	7.515e-3	3	NC	15	NC	3
84		min	-.394	4	-1.134	2	-.026	5	-1.208e-2	2	267.291	2	3502.905	1
85	5	max	0	12	.911	3	.066	1	8.354e-3	3	NC	15	NC	3
86		min	-.394	4	-1.466	2	-.009	5	-1.334e-2	2	227.351	2	2910	1
87	6	max	0	12	.964	3	.065	1	9.193e-3	3	NC	15	NC	3
88		min	-.394	4	-1.524	2	0	10	-1.459e-2	2	212.779	2	2987.116	1
89	7	max	0	12	.984	3	.05	1	1.003e-2	3	9976.556	15	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-.394	4	-1.522	2	-.003	10	-1.585e-2	2	213.287	2	3840.067	1
91		8	max	0	12	.981	3	.042	4	1.087e-2	3	NC	15	NC	2
92			min	-.394	4	-1.479	2	-.006	10	-1.711e-2	2	224.121	2	4417.26	4
93		9	max	0	12	.966	3	.03	4	1.171e-2	3	NC	15	NC	1
94			min	-.394	4	-1.423	2	-.01	2	-1.837e-2	2	239.793	2	6099.26	4
95		10	max	0	1	.957	3	.021	3	1.255e-2	3	NC	15	NC	1
96			min	-.394	4	-1.394	1	-.015	2	-1.963e-2	2	248.646	1	NC	1
97		11	max	0	1	.966	3	.021	3	1.171e-2	3	NC	15	NC	1
98			min	-.394	4	-1.423	2	-.026	5	-1.837e-2	2	239.793	2	7849.732	5
99		12	max	0	1	.981	3	.028	1	1.087e-2	3	NC	15	NC	2
100			min	-.394	4	-1.479	2	-.031	5	-1.711e-2	2	224.121	2	6558.584	5
101		13	max	0	1	.984	3	.05	1	1.003e-2	3	9976.43	15	NC	2
102			min	-.394	4	-1.522	2	-.021	5	-1.585e-2	2	213.287	2	3840.067	1
103		14	max	0	1	.964	3	.065	1	9.193e-3	3	NC	15	NC	3
104			min	-.394	4	-1.524	2	-.003	5	-1.459e-2	2	212.779	2	2987.116	1
105		15	max	0	1	.911	3	.066	1	8.354e-3	3	NC	15	NC	3
106			min	-.394	4	-1.466	2	0	10	-1.334e-2	2	227.351	2	2910	1
107		16	max	0	1	.825	3	.055	1	7.515e-3	3	NC	15	NC	3
108			min	-.394	4	-1.34	2	0	10	-1.208e-2	2	267.291	2	3502.905	1
109		17	max	0	1	.706	3	.047	4	6.676e-3	3	NC	5	NC	2
110			min	-.394	4	-1.148	2	0	10	-1.082e-2	2	365.242	2	3989.317	4
111		18	max	0	1	.56	3	.033	4	5.837e-3	3	NC	5	NC	1
112			min	-.394	4	-.901	2	-.002	10	-9.562e-3	2	688.76	2	5703.507	4
113		19	max	0	1	.4	3	.007	3	4.997e-3	3	NC	1	NC	1
114			min	-.394	4	-.622	2	-.004	2	-8.304e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.197	1	.006	3	9.43e-3	3	NC	1	NC	1
116			min	-.133	4	-.141	3	-.004	2	-1.282e-2	1	NC	1	NC	1
117		2	max	0	12	.08	1	.019	1	1.04e-2	3	NC	4	NC	2
118			min	-.133	4	-.096	3	-.019	5	-1.365e-2	1	1458.58	2	9917.294	1
119		3	max	0	12	.013	9	.045	1	1.137e-2	3	NC	5	NC	2
120			min	-.133	4	-.063	3	-.024	5	-1.449e-2	1	816.779	2	4242.55	1
121		4	max	0	12	.004	13	.067	1	1.234e-2	3	NC	5	NC	3
122			min	-.133	4	-.103	2	-.02	5	-1.532e-2	1	658.631	2	2871.936	1
123		5	max	0	12	.005	4	.077	1	1.331e-2	3	NC	5	NC	3
124			min	-.133	4	-.104	2	-.009	5	-1.615e-2	1	656.541	2	2485.525	1
125		6	max	0	12	.021	9	.074	1	1.429e-2	3	NC	5	NC	3
126			min	-.133	4	-.097	3	.001	10	-1.699e-2	1	801.355	2	2613.839	1
127		7	max	0	12	.086	1	.057	1	1.526e-2	3	NC	3	NC	2
128			min	-.133	4	-.149	3	-.001	10	-1.782e-2	1	1324.349	2	3402.866	1
129		8	max	0	12	.193	1	.031	1	1.623e-2	3	NC	1	NC	2
130			min	-.133	4	-.208	3	-.004	10	-1.865e-2	1	2875.288	3	6189.095	1
131		9	max	0	12	.288	1	.019	4	1.72e-2	3	NC	4	NC	1
132			min	-.133	4	-.258	3	-.009	2	-1.949e-2	1	1641.557	3	9781.676	4
133		10	max	0	1	.33	1	.018	3	1.817e-2	3	NC	5	NC	1
134			min	-.133	4	-.28	3	-.014	2	-2.032e-2	1	1381.211	3	NC	1
135		11	max	0	1	.288	1	.019	3	1.72e-2	3	NC	4	NC	1
136			min	-.133	4	-.258	3	-.014	5	-1.949e-2	1	1641.557	3	NC	1
137		12	max	0	1	.193	1	.031	1	1.623e-2	3	NC	1	NC	2
138			min	-.133	4	-.208	3	-.015	5	-1.865e-2	1	2875.288	3	6189.095	1
139		13	max	0	1	.086	1	.057	1	1.526e-2	3	NC	3	NC	2
140			min	-.133	4	-.149	3	-.007	5	-1.782e-2	1	1324.349	2	3402.866	1
141		14	max	0	1	.021	9	.074	1	1.429e-2	3	NC	5	NC	3
142			min	-.133	4	-.097	3	.001	10	-1.699e-2	1	801.355	2	2613.839	1
143		15	max	0	1	.005	6	.077	1	1.331e-2	3	NC	5	NC	3
144			min	-.133	4	-.104	2	.002	10	-1.615e-2	1	656.541	2	2485.525	1
145		16	max	0	1	.004	13	.067	1	1.234e-2	3	NC	5	NC	3
146			min	-.133	4	-.103	2	.002	10	-1.532e-2	1	658.631	2	2871.936	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.013	9	.045	1	1.137e-2	3	NC	5	NC	2
148			min	-1.133	4	-.063	3	0	10	-1.449e-2	1	816.779	2	4242.55	1
149		18	max	0	1	.08	1	.026	4	1.04e-2	3	NC	4	NC	2
150			min	-1.133	4	-.096	3	-.001	10	-1.365e-2	1	1458.58	2	7104.855	4
151		19	max	0	1	.197	1	.006	3	9.43e-3	3	NC	1	NC	1
152			min	-1.133	4	-.141	3	-.004	2	-1.282e-2	1	NC	1	NC	1
153	M2	1	max	.006	1	.007	2	.008	1	2.295e-3	5	NC	1	NC	2
154			min	-.008	3	-.012	3	-.589	4	-1.683e-4	1	8314.684	2	102.842	4
155		2	max	.006	1	.006	2	.007	1	2.297e-3	5	NC	1	NC	2
156			min	-.007	3	-.011	3	-.54	4	-1.577e-4	1	9618.018	2	112.069	4
157		3	max	.006	1	.005	2	.006	1	2.299e-3	5	NC	1	NC	2
158			min	-.007	3	-.011	3	-.492	4	-1.471e-4	1	NC	1	123.045	4
159		4	max	.005	1	.004	2	.006	1	2.301e-3	5	NC	1	NC	1
160			min	-.006	3	-.011	3	-.445	4	-1.365e-4	1	NC	1	136.228	4
161		5	max	.005	1	.003	2	.005	1	2.303e-3	5	NC	1	NC	1
162			min	-.006	3	-.01	3	-.398	4	-1.259e-4	1	NC	1	152.245	4
163		6	max	.005	1	.003	2	.005	1	2.305e-3	5	NC	1	NC	1
164			min	-.005	3	-.01	3	-.352	4	-1.152e-4	1	NC	1	171.966	4
165		7	max	.004	1	.002	2	.004	1	2.309e-3	4	NC	1	NC	1
166			min	-.005	3	-.009	3	-.308	4	-1.046e-4	1	NC	1	196.63	4
167		8	max	.004	1	0	2	.003	1	2.313e-3	4	NC	1	NC	1
168			min	-.005	3	-.009	3	-.266	4	-9.399e-5	1	NC	1	228.049	4
169		9	max	.004	1	0	2	.003	1	2.318e-3	4	NC	1	NC	1
170			min	-.004	3	-.008	3	-.225	4	-8.337e-5	1	NC	1	268.964	4
171		10	max	.003	1	0	2	.002	1	2.323e-3	4	NC	1	NC	1
172			min	-.004	3	-.008	3	-.187	4	-7.275e-5	1	NC	1	323.679	4
173		11	max	.003	1	0	15	.002	1	2.328e-3	4	NC	1	NC	1
174			min	-.003	3	-.007	3	-.152	4	-6.213e-5	1	NC	1	399.276	4
175		12	max	.003	1	0	15	.002	1	2.332e-3	4	NC	1	NC	1
176			min	-.003	3	-.006	3	-.119	4	-5.15e-5	1	NC	1	508.097	4
177		13	max	.002	1	0	15	.001	1	2.337e-3	4	NC	1	NC	1
178			min	-.003	3	-.006	3	-.09	4	-4.088e-5	1	NC	1	673.262	4
179		14	max	.002	1	0	15	0	1	2.342e-3	4	NC	1	NC	1
180			min	-.002	3	-.005	3	-.064	4	-3.026e-5	1	NC	1	942.383	4
181		15	max	.001	1	0	15	0	1	2.347e-3	4	NC	1	NC	1
182			min	-.002	3	-.004	3	-.042	4	-1.964e-5	1	NC	1	1427.077	4
183		16	max	.001	1	0	15	0	1	2.351e-3	4	NC	1	NC	1
184			min	-.001	3	-.003	3	-.025	4	-9.018e-6	1	NC	1	2443.633	4
185		17	max	0	1	0	15	0	1	2.356e-3	4	NC	1	NC	1
186			min	0	3	-.002	3	-.012	4	-7.129e-7	3	NC	1	5213.961	4
187		18	max	0	1	0	15	0	1	2.361e-3	4	NC	1	NC	1
188			min	0	3	-.001	3	-.003	4	2.936e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.366e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.03e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-3.517e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.571e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	1.715e-4	4	NC	1	NC	1
194			min	0	2	-.002	6	0	12	8.816e-7	12	NC	1	NC	1
195		3	max	0	3	-.001	15	.026	4	8.001e-4	4	NC	1	NC	1
196			min	0	2	-.005	6	0	12	2.115e-6	12	NC	1	NC	1
197		4	max	.001	3	-.002	15	.038	4	1.429e-3	4	NC	1	NC	1
198			min	0	2	-.008	6	0	12	3.348e-6	12	NC	1	NC	1
199		5	max	.001	3	-.002	15	.05	4	2.057e-3	4	NC	1	NC	1
200			min	-.001	2	-.011	6	0	12	4.582e-6	12	9481.517	6	NC	1
201		6	max	.002	3	-.003	15	.062	4	2.686e-3	4	NC	1	NC	1
202			min	-.001	2	-.013	6	0	12	5.815e-6	12	7593.352	6	NC	1
203		7	max	.002	3	-.003	15	.073	4	3.314e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.016	6	0	12	7.048e-6	12	6461.591	6	9847.687	5
205		8	max	.003	3	-.004	15	.083	4	3.943e-3	4	NC	5	NC	1
206			min	-.002	2	-.018	6	0	12	8.282e-6	12	5762.959	6	9695.606	5
207		9	max	.003	3	-.004	15	.093	4	4.572e-3	4	NC	5	NC	1
208			min	-.002	2	-.019	6	0	12	9.515e-6	12	5345.763	6	9959.087	5
209		10	max	.003	3	-.004	15	.103	4	5.2e-3	4	NC	5	NC	1
210			min	-.003	2	-.02	6	0	12	1.075e-5	12	5135.991	6	NC	1
211		11	max	.004	3	-.004	15	.112	4	5.829e-3	4	NC	5	NC	1
212			min	-.003	2	-.02	6	0	12	1.198e-5	12	5102.24	6	NC	1
213		12	max	.004	3	-.004	15	.121	4	6.457e-3	4	NC	5	NC	1
214			min	-.003	2	-.02	6	0	12	1.321e-5	12	5243.59	6	NC	1
215		13	max	.004	3	-.004	15	.13	4	7.086e-3	4	NC	5	NC	1
216			min	-.004	2	-.018	6	0	12	1.445e-5	12	5590.741	6	NC	1
217		14	max	.005	3	-.004	15	.139	4	7.715e-3	4	NC	5	NC	1
218			min	-.004	2	-.017	6	0	12	1.568e-5	12	6222.34	6	NC	1
219		15	max	.005	3	-.003	15	.148	4	8.343e-3	4	NC	2	NC	1
220			min	-.004	2	-.014	6	0	12	1.691e-5	12	7314.35	6	NC	1
221		16	max	.006	3	-.002	15	.157	4	8.972e-3	4	NC	1	NC	1
222			min	-.004	2	-.011	6	0	12	1.815e-5	12	9294.198	6	NC	1
223		17	max	.006	3	-.001	15	.167	4	9.6e-3	4	NC	1	NC	1
224			min	-.005	2	-.008	1	0	12	1.938e-5	12	NC	1	NC	1
225		18	max	.006	3	0	15	.177	4	1.023e-2	4	NC	1	NC	1
226			min	-.005	2	-.005	1	0	12	2.061e-5	12	NC	1	NC	1
227		19	max	.007	3	0	5	.187	4	1.086e-2	4	NC	1	NC	1
228			min	-.005	2	-.002	1	0	12	2.185e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.005	2	0	12	6.258e-5	1	NC	1	NC	3
230			min	0	3	-.007	3	-.187	4	-7.343e-4	5	NC	1	132.428	4
231		2	max	.003	1	.005	2	0	12	6.258e-5	1	NC	1	NC	2
232			min	0	3	-.006	3	-.172	4	-7.343e-4	5	NC	1	144.172	4
233		3	max	.002	1	.004	2	0	12	6.258e-5	1	NC	1	NC	2
234			min	0	3	-.006	3	-.157	4	-7.343e-4	5	NC	1	158.14	4
235		4	max	.002	1	.004	2	0	12	6.258e-5	1	NC	1	NC	2
236			min	0	3	-.006	3	-.142	4	-7.343e-4	5	NC	1	174.909	4
237		5	max	.002	1	.004	2	0	12	6.258e-5	1	NC	1	NC	2
238			min	0	3	-.005	3	-.127	4	-7.343e-4	5	NC	1	195.267	4
239		6	max	.002	1	.003	2	0	12	6.258e-5	1	NC	1	NC	2
240			min	0	3	-.005	3	-.113	4	-7.343e-4	5	NC	1	220.306	4
241		7	max	.002	1	.003	2	0	12	6.258e-5	1	NC	1	NC	2
242			min	0	3	-.004	3	-.099	4	-7.343e-4	5	NC	1	251.573	4
243		8	max	.002	1	.003	2	0	12	6.258e-5	1	NC	1	NC	2
244			min	0	3	-.004	3	-.085	4	-7.343e-4	5	NC	1	291.329	4
245		9	max	.002	1	.003	2	0	12	6.258e-5	1	NC	1	NC	2
246			min	0	3	-.004	3	-.072	4	-7.343e-4	5	NC	1	342.976	4
247		10	max	.001	1	.002	2	0	12	6.258e-5	1	NC	1	NC	1
248			min	0	3	-.003	3	-.06	4	-7.343e-4	5	NC	1	411.832	4
249		11	max	.001	1	.002	2	0	12	6.258e-5	1	NC	1	NC	1
250			min	0	3	-.003	3	-.049	4	-7.343e-4	5	NC	1	506.599	4
251		12	max	.001	1	.002	2	0	12	6.258e-5	1	NC	1	NC	1
252			min	0	3	-.003	3	-.039	4	-7.343e-4	5	NC	1	642.33	4
253		13	max	0	1	.002	2	0	12	6.258e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.029	4	-7.343e-4	5	NC	1	846.973	4
255		14	max	0	1	.001	2	0	12	6.258e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	-.021	4	-7.343e-4	5	NC	1	1177.392	4
257		15	max	0	1	.001	2	0	12	6.258e-5	1	NC	1	NC	1
258			min	0	3	-.001	3	-.014	4	-7.343e-4	5	NC	1	1764.697	4
259		16	max	0	1	0	2	0	12	6.258e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	-.008	4	-7.343e-4	5	NC	1	2971.422	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	6.258e-5	1	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-7.343e-4	5	NC	1	6142.237	4
263		18	max	0	1	0	2	0	12	6.258e-5	1	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-7.343e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.258e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-7.343e-4	5	NC	1	NC	1
267	M6	1	max	.02	1	.025	2	0	1	2.389e-3	4	NC	3	NC	1
268			min	-.024	3	-.036	3	-.595	4	0	1	2384.47	2	101.831	4
269		2	max	.019	1	.023	2	0	1	2.388e-3	4	NC	3	NC	1
270			min	-.023	3	-.034	3	-.546	4	0	1	2609.481	2	110.969	4
271		3	max	.018	1	.021	2	0	1	2.387e-3	4	NC	3	NC	1
272			min	-.021	3	-.032	3	-.497	4	0	1	2879.38	2	121.837	4
273		4	max	.017	1	.019	2	0	1	2.386e-3	4	NC	3	NC	1
274			min	-.02	3	-.03	3	-.449	4	0	1	3206.586	2	134.892	4
275		5	max	.015	1	.017	2	0	1	2.385e-3	4	NC	3	NC	1
276			min	-.019	3	-.028	3	-.402	4	0	1	3608.26	2	150.753	4
277		6	max	.014	1	.015	2	0	1	2.385e-3	4	NC	3	NC	1
278			min	-.017	3	-.026	3	-.356	4	0	1	4108.67	2	170.282	4
279		7	max	.013	1	.013	2	0	1	2.384e-3	4	NC	3	NC	1
280			min	-.016	3	-.024	3	-.311	4	0	1	4743.053	2	194.706	4
281		8	max	.012	1	.011	2	0	1	2.383e-3	4	NC	1	NC	1
282			min	-.015	3	-.022	3	-.268	4	0	1	5564.181	2	225.82	4
283		9	max	.011	1	.009	2	0	1	2.382e-3	4	NC	1	NC	1
284			min	-.013	3	-.02	3	-.227	4	0	1	6654.054	2	266.338	4
285		10	max	.01	1	.007	2	0	1	2.381e-3	4	NC	1	NC	1
286			min	-.012	3	-.018	3	-.189	4	0	1	8145.853	2	320.522	4
287		11	max	.009	1	.006	2	0	1	2.38e-3	4	NC	1	NC	1
288			min	-.011	3	-.016	3	-.153	4	0	1	NC	1	395.387	4
289		12	max	.008	1	.005	2	0	1	2.38e-3	4	NC	1	NC	1
290			min	-.009	3	-.014	3	-.12	4	0	1	NC	1	503.155	4
291		13	max	.007	1	.003	2	0	1	2.379e-3	4	NC	1	NC	1
292			min	-.008	3	-.012	3	-.091	4	0	1	NC	1	666.723	4
293		14	max	.006	1	.002	2	0	1	2.378e-3	4	NC	1	NC	1
294			min	-.007	3	-.01	3	-.065	4	0	1	NC	1	933.249	4
295		15	max	.004	1	.001	2	0	1	2.377e-3	4	NC	1	NC	1
296			min	-.005	3	-.008	3	-.043	4	0	1	NC	1	1413.277	4
297		16	max	.003	1	0	2	0	1	2.376e-3	4	NC	1	NC	1
298			min	-.004	3	-.006	3	-.025	4	0	1	NC	1	2420.081	4
299		17	max	.002	1	0	2	0	1	2.375e-3	4	NC	1	NC	1
300			min	-.003	3	-.004	3	-.012	4	0	1	NC	1	5163.982	4
301		18	max	.001	1	0	2	0	1	2.374e-3	4	NC	1	NC	1
302			min	-.001	3	-.002	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.374e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-4.579e-4	4	NC	1	NC	1
307		2	max	.001	3	0	15	.013	4	1.538e-4	4	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309		3	max	.002	3	-.001	15	.026	4	7.654e-4	4	NC	1	NC	1
310			min	-.002	2	-.007	3	0	1	0	1	NC	1	NC	1
311		4	max	.003	3	-.002	15	.038	4	1.377e-3	4	NC	1	NC	1
312			min	-.003	2	-.01	3	0	1	0	1	NC	1	NC	1
313		5	max	.004	3	-.003	15	.05	4	1.989e-3	4	NC	1	NC	1
314			min	-.004	2	-.013	3	0	1	0	1	8548.098	3	NC	1
315		6	max	.006	3	-.003	15	.062	4	2.6e-3	4	NC	1	NC	1
316			min	-.005	2	-.015	3	0	1	0	1	7183.938	3	9117.441	4
317		7	max	.007	3	-.004	15	.072	4	3.212e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.006	2	-.017	3	0	1	0	1	6363.011	3	8478.626	4
319	8	max	.008	3	-.004	15	.083	4	3.824e-3	4	NC	2	NC	1
320		min	-.007	2	-.019	3	0	1	0	1	5770.692	4	8243.699	4
321	9	max	.009	3	-.005	15	.093	4	4.435e-3	4	NC	2	NC	1
322		min	-.008	2	-.02	4	0	1	0	1	5352.437	4	8337.641	4
323	10	max	.01	3	-.005	15	.102	4	5.047e-3	4	NC	5	NC	1
324		min	-.009	2	-.021	4	0	1	0	1	5142.006	4	8758.156	4
325	11	max	.011	3	-.005	15	.111	4	5.659e-3	4	NC	5	NC	1
326		min	-.01	2	-.021	4	0	1	0	1	5107.884	4	9567.002	4
327	12	max	.012	3	-.005	15	.12	4	6.27e-3	4	NC	5	NC	1
328		min	-.011	2	-.02	4	0	1	0	1	5249.107	4	NC	1
329	13	max	.013	3	-.004	15	.129	4	6.882e-3	4	NC	2	NC	1
330		min	-.012	2	-.019	4	0	1	0	1	5596.371	4	NC	1
331	14	max	.014	3	-.004	15	.137	4	7.493e-3	4	NC	2	NC	1
332		min	-.013	2	-.017	4	0	1	0	1	6228.373	4	NC	1
333	15	max	.016	3	-.003	15	.145	4	8.105e-3	4	NC	1	NC	1
334		min	-.014	2	-.015	4	0	1	0	1	7321.22	4	NC	1
335	16	max	.017	3	-.003	15	.154	4	8.717e-3	4	NC	1	NC	1
336		min	-.015	2	-.012	4	0	1	0	1	9302.705	4	NC	1
337	17	max	.018	3	-.002	15	.162	4	9.328e-3	4	NC	1	NC	1
338		min	-.016	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.019	3	-.001	15	.172	4	9.94e-3	4	NC	1	NC	1
340		min	-.017	2	-.008	1	0	1	0	1	NC	1	NC	1
341	19	max	.02	3	0	15	.181	4	1.055e-2	4	NC	1	NC	1
342		min	-.018	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.017	2	0	0	1	NC	1	NC	1
344		min	-.002	3	-.02	3	-.181	4	-8.383e-4	4	NC	1	136.814	4
345	2	max	.007	1	.016	2	0	1	0	1	NC	1	NC	1
346		min	-.002	3	-.019	3	-.167	4	-8.383e-4	4	NC	1	148.96	4
347	3	max	.007	1	.015	2	0	1	0	1	NC	1	NC	1
348		min	-.002	3	-.018	3	-.152	4	-8.383e-4	4	NC	1	163.403	4
349	4	max	.006	1	.014	2	0	1	0	1	NC	1	NC	1
350		min	-.002	3	-.017	3	-.137	4	-8.383e-4	4	NC	1	180.743	4
351	5	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
352		min	-.002	3	-.016	3	-.123	4	-8.383e-4	4	NC	1	201.793	4
353	6	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
354		min	-.001	3	-.015	3	-.109	4	-8.383e-4	4	NC	1	227.683	4
355	7	max	.005	1	.011	2	0	1	0	1	NC	1	NC	1
356		min	-.001	3	-.013	3	-.095	4	-8.383e-4	4	NC	1	260.012	4
357	8	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
358		min	-.001	3	-.012	3	-.082	4	-8.383e-4	4	NC	1	301.119	4
359	9	max	.004	1	.01	2	0	1	0	1	NC	1	NC	1
360		min	-.001	3	-.011	3	-.07	4	-8.383e-4	4	NC	1	354.521	4
361	10	max	.004	1	.009	2	0	1	0	1	NC	1	NC	1
362		min	-.001	3	-.01	3	-.058	4	-8.383e-4	4	NC	1	425.717	4
363	11	max	.003	1	.008	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.009	3	-.047	4	-8.383e-4	4	NC	1	523.704	4
365	12	max	.003	1	.007	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.008	3	-.037	4	-8.383e-4	4	NC	1	664.05	4
367	13	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.007	3	-.028	4	-8.383e-4	4	NC	1	875.654	4
369	14	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.006	3	-.02	4	-8.383e-4	4	NC	1	1217.318	4
371	15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.004	3	-.014	4	-8.383e-4	4	NC	1	1824.625	4
373	16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.003	3	-.008	4	-8.383e-4	4	NC	1	3072.481	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.004	4	-8.383e-4	4	NC	1	6351.504	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-8.383e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-8.383e-4	4	NC	1	NC	1
381	M10	1	max	.006	1	.007	2	0	12	2.37e-3	4	NC	1	NC	2
382			min	-.008	3	-.012	3	-.593	4	1.223e-5	12	8314.684	2	102.136	4
383		2	max	.006	1	.006	2	0	12	2.369e-3	4	NC	1	NC	2
384			min	-.007	3	-.011	3	-.544	4	1.15e-5	12	9618.018	2	111.301	4
385		3	max	.006	1	.005	2	0	12	2.368e-3	4	NC	1	NC	2
386			min	-.007	3	-.011	3	-.496	4	1.076e-5	12	NC	1	122.202	4
387		4	max	.005	1	.004	2	0	12	2.367e-3	4	NC	1	NC	1
388			min	-.006	3	-.011	3	-.448	4	1.002e-5	12	NC	1	135.295	4
389		5	max	.005	1	.003	2	0	12	2.366e-3	4	NC	1	NC	1
390			min	-.006	3	-.01	3	-.401	4	9.286e-6	12	NC	1	151.204	4
391		6	max	.005	1	.003	2	0	12	2.365e-3	4	NC	1	NC	1
392			min	-.005	3	-.01	3	-.355	4	8.549e-6	12	NC	1	170.791	4
393		7	max	.004	1	.002	2	0	12	2.364e-3	4	NC	1	NC	1
394			min	-.005	3	-.009	3	-.31	4	7.812e-6	12	NC	1	195.288	4
395		8	max	.004	1	0	2	0	12	2.363e-3	4	NC	1	NC	1
396			min	-.005	3	-.009	3	-.267	4	7.e-6	10	NC	1	226.495	4
397		9	max	.004	1	0	2	0	12	2.362e-3	4	NC	1	NC	1
398			min	-.004	3	-.008	3	-.227	4	6.18e-6	10	NC	1	267.134	4
399		10	max	.003	1	0	2	0	12	2.361e-3	4	NC	1	NC	1
400			min	-.004	3	-.008	3	-.188	4	5.361e-6	10	NC	1	321.48	4
401		11	max	.003	1	0	2	0	12	2.361e-3	4	NC	1	NC	1
402			min	-.003	3	-.007	3	-.153	4	4.541e-6	10	NC	1	396.569	4
403		12	max	.003	1	-.001	2	0	12	2.36e-3	4	NC	1	NC	1
404			min	-.003	3	-.006	3	-.12	4	3.722e-6	10	NC	1	504.66	4
405		13	max	.002	1	-.001	15	0	12	2.359e-3	4	NC	1	NC	1
406			min	-.003	3	-.006	3	-.091	4	2.902e-6	10	NC	1	668.72	4
407		14	max	.002	1	-.001	15	0	12	2.358e-3	4	NC	1	NC	1
408			min	-.002	3	-.005	3	-.065	4	2.083e-6	10	NC	1	936.049	4
409		15	max	.001	1	-.001	15	0	12	2.357e-3	4	NC	1	NC	1
410			min	-.002	3	-.004	3	-.043	4	1.263e-6	10	NC	1	1417.53	4
411		16	max	.001	1	0	15	0	12	2.356e-3	4	NC	1	NC	1
412			min	-.001	3	-.003	4	-.025	4	4.435e-7	10	NC	1	2427.404	4
413		17	max	0	1	0	15	0	12	2.355e-3	4	NC	1	NC	1
414			min	0	3	-.002	4	-.012	4	-1.603e-6	1	NC	1	5179.784	4
415		18	max	0	1	0	15	0	12	2.354e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.003	4	-1.222e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.353e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.285e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	7.045e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.533e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	1.635e-4	4	NC	1	NC	1
422			min	0	2	-.003	4	0	1	-1.491e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	.026	4	7.804e-4	4	NC	1	NC	1
424			min	0	2	-.006	4	0	1	-3.687e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	.038	4	1.397e-3	4	NC	1	NC	1
426			min	0	2	-.009	4	0	1	-5.883e-5	1	NC	1	NC	1
427		5	max	.001	3	-.003	15	.05	4	2.014e-3	4	NC	1	NC	1
428			min	-.001	2	-.012	4	0	1	-8.079e-5	1	9033.107	4	NC	1
429		6	max	.002	3	-.004	15	.061	4	2.631e-3	4	NC	1	NC	1
430			min	-.001	2	-.014	4	0	1	-1.028e-4	1	7268.436	4	NC	1
431		7	max	.002	3	-.004	15	.072	4	3.248e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.017	4	0	1	-1.247e-4	1	6208.43	4	9448.407	4
433		8	max	.003	3	-.005	15	.082	4	3.865e-3	4	NC	5	NC	1
434			min	-.002	2	-.019	4	-.001	1	-1.467e-4	1	5554.138	4	9273.767	4
435		9	max	.003	3	-.005	15	.092	4	4.481e-3	4	NC	5	NC	1
436			min	-.002	2	-.02	4	-.001	1	-1.686e-4	1	5165.064	4	9488.439	4
437		10	max	.003	3	-.005	15	.102	4	5.098e-3	4	NC	5	NC	1
438			min	-.003	2	-.021	4	-.002	1	-1.906e-4	1	4972.8	4	NC	1
439		11	max	.004	3	-.005	15	.111	4	5.715e-3	4	NC	5	NC	1
440			min	-.003	2	-.021	4	-.002	1	-2.125e-4	1	4948.797	4	NC	1
441		12	max	.004	3	-.005	15	.12	4	6.332e-3	4	NC	5	NC	1
442			min	-.003	2	-.021	4	-.003	1	-2.345e-4	1	5093.384	4	NC	1
443		13	max	.004	3	-.005	15	.129	4	6.949e-3	4	NC	5	NC	1
444			min	-.004	2	-.02	4	-.003	1	-2.565e-4	1	5437.274	4	NC	1
445		14	max	.005	3	-.004	15	.137	4	7.566e-3	4	NC	5	NC	1
446			min	-.004	2	-.018	4	-.004	1	-2.784e-4	1	6057.704	4	NC	1
447		15	max	.005	3	-.004	15	.146	4	8.182e-3	4	NC	2	NC	1
448			min	-.004	2	-.015	4	-.004	1	-3.004e-4	1	7126.722	4	NC	1
449		16	max	.006	3	-.003	15	.155	4	8.799e-3	4	NC	1	NC	1
450			min	-.004	2	-.012	4	-.005	1	-3.223e-4	1	9061.67	4	NC	1
451		17	max	.006	3	-.002	15	.164	4	9.416e-3	4	NC	1	NC	1
452			min	-.005	2	-.009	4	-.005	1	-3.443e-4	1	NC	1	NC	1
453		18	max	.006	3	-.001	15	.173	4	1.003e-2	4	NC	1	NC	1
454			min	-.005	2	-.005	1	-.006	1	-3.663e-4	1	NC	1	NC	1
455		19	max	.007	3	0	10	.183	4	1.065e-2	4	NC	1	NC	1
456			min	-.005	2	-.002	1	-.007	1	-3.882e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.005	2	.007	1	-4.068e-6	12	NC	1	NC	3
458			min	0	3	-.007	3	-.183	4	-7.627e-4	4	NC	1	135.193	4
459		2	max	.003	1	.005	2	.007	1	-4.068e-6	12	NC	1	NC	2
460			min	0	3	-.006	3	-.169	4	-7.627e-4	4	NC	1	147.186	4
461		3	max	.002	1	.004	2	.006	1	-4.068e-6	12	NC	1	NC	2
462			min	0	3	-.006	3	-.154	4	-7.627e-4	4	NC	1	161.448	4
463		4	max	.002	1	.004	2	.005	1	-4.068e-6	12	NC	1	NC	2
464			min	0	3	-.006	3	-.139	4	-7.627e-4	4	NC	1	178.572	4
465		5	max	.002	1	.004	2	.005	1	-4.068e-6	12	NC	1	NC	2
466			min	0	3	-.005	3	-.124	4	-7.627e-4	4	NC	1	199.36	4
467		6	max	.002	1	.003	2	.004	1	-4.068e-6	12	NC	1	NC	2
468			min	0	3	-.005	3	-.11	4	-7.627e-4	4	NC	1	224.926	4
469		7	max	.002	1	.003	2	.004	1	-4.068e-6	12	NC	1	NC	2
470			min	0	3	-.004	3	-.097	4	-7.627e-4	4	NC	1	256.854	4
471		8	max	.002	1	.003	2	.003	1	-4.068e-6	12	NC	1	NC	2
472			min	0	3	-.004	3	-.083	4	-7.627e-4	4	NC	1	297.449	4
473		9	max	.002	1	.003	2	.003	1	-4.068e-6	12	NC	1	NC	2
474			min	0	3	-.004	3	-.071	4	-7.627e-4	4	NC	1	350.186	4
475		10	max	.001	1	.002	2	.002	1	-4.068e-6	12	NC	1	NC	1
476			min	0	3	-.003	3	-.059	4	-7.627e-4	4	NC	1	420.495	4
477		11	max	.001	1	.002	2	.002	1	-4.068e-6	12	NC	1	NC	1
478			min	0	3	-.003	3	-.048	4	-7.627e-4	4	NC	1	517.261	4
479		12	max	.001	1	.002	2	.001	1	-4.068e-6	12	NC	1	NC	1
480			min	0	3	-.003	3	-.038	4	-7.627e-4	4	NC	1	655.857	4
481		13	max	0	1	.002	2	.001	1	-4.068e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.029	4	-7.627e-4	4	NC	1	864.82	4
483		14	max	0	1	.001	2	0	1	-4.068e-6	12	NC	1	NC	1
484			min	0	3	-.002	3	-.021	4	-7.627e-4	4	NC	1	1202.216	4
485		15	max	0	1	.001	2	0	1	-4.068e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.014	4	-7.627e-4	4	NC	1	1801.925	4
487		16	max	0	1	0	2	0	1	-4.068e-6	12	NC	1	NC	1
488			min	0	3	-.001	3	-.008	4	-7.627e-4	4	NC	1	3034.145	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-4.068e-6	12	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-7.627e-4	4	NC	1	6271.984	4
491		18	max	0	1	0	2	0	1	-4.068e-6	12	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-7.627e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-4.068e-6	12	NC	1	NC	1
494			min	0	1	0	1	0	1	-7.627e-4	4	NC	1	NC	1
495	M1	1	max	.009	3	.211	2	.625	4	9.048e-3	1	NC	1	NC	1
496			min	-.005	2	-.054	3	0	12	-1.702e-2	3	NC	1	NC	1
497		2	max	.009	3	.104	2	.607	4	8.06e-3	4	NC	5	NC	1
498			min	-.005	2	-.027	3	-.005	1	-8.449e-3	3	1266.902	2	NC	1
499		3	max	.009	3	.012	3	.589	4	1.433e-2	4	NC	5	NC	1
500			min	-.005	2	-.01	2	-.008	1	-1.556e-4	1	613.171	2	6833.868	5
501		4	max	.008	3	.073	3	.569	4	1.242e-2	4	NC	15	NC	1
502			min	-.005	2	-.137	2	-.007	1	-3.976e-3	3	389.855	2	4842.513	5
503		5	max	.008	3	.148	3	.55	4	1.05e-2	4	NC	15	NC	1
504			min	-.005	2	-.268	2	-.005	1	-7.859e-3	3	282.95	2	3824.96	5
505		6	max	.008	3	.229	3	.53	4	1.295e-2	1	8003.101	15	NC	1
506			min	-.005	2	-.394	2	-.002	1	-1.174e-2	3	223.8	2	3205.774	5
507		7	max	.008	3	.306	3	.509	4	1.733e-2	1	6763.747	15	NC	1
508			min	-.004	2	-.507	2	0	3	-1.562e-2	3	188.768	2	2779.473	4
509		8	max	.008	3	.37	3	.488	4	2.17e-2	1	6029.34	15	NC	1
510			min	-.004	2	-.596	2	0	12	-1.951e-2	3	168.001	2	2464.679	4
511		9	max	.008	3	.412	3	.466	4	2.384e-2	1	5644.617	15	NC	1
512			min	-.004	2	-.652	2	0	1	-1.999e-2	3	157.014	1	2258.137	4
513		10	max	.007	3	.428	3	.44	4	2.459e-2	2	5526.918	15	NC	1
514			min	-.004	2	-.671	2	0	10	-1.82e-2	3	153.711	1	2191.202	4
515		11	max	.007	3	.418	3	.412	4	2.592e-2	2	5644.388	15	NC	1
516			min	-.004	2	-.652	2	0	12	-1.641e-2	3	157.263	1	2228.669	4
517		12	max	.007	3	.383	3	.381	4	2.475e-2	2	6028.791	15	NC	1
518			min	-.004	2	-.594	2	0	1	-1.42e-2	3	168.795	1	2375.282	4
519		13	max	.007	3	.326	3	.344	4	1.985e-2	2	6762.668	15	NC	1
520			min	-.004	2	-.502	1	0	1	-1.136e-2	3	190.99	1	2813.214	4
521		14	max	.007	3	.254	3	.305	4	1.495e-2	2	8001.119	15	NC	1
522			min	-.004	2	-.386	1	0	12	-8.525e-3	3	228.722	1	3801.775	4
523		15	max	.006	3	.172	3	.264	4	1.004e-2	2	NC	15	NC	1
524			min	-.004	2	-.258	1	0	12	-5.688e-3	3	293.117	1	6187.29	4
525		16	max	.006	3	.087	3	.224	4	8.948e-3	4	NC	15	NC	1
526			min	-.004	2	-.127	1	0	12	-2.851e-3	3	411.011	1	NC	1
527		17	max	.006	3	.004	3	.188	4	1.008e-2	4	NC	5	NC	1
528			min	-.004	2	-.006	2	0	12	-1.391e-5	3	659.93	1	NC	1
529		18	max	.006	3	.101	1	.158	4	6.46e-3	2	NC	5	NC	1
530			min	-.004	2	-.071	3	0	12	-2.202e-3	3	1384.091	1	NC	1
531		19	max	.006	3	.197	1	.133	4	1.288e-2	2	NC	1	NC	1
532			min	-.004	2	-.141	3	0	1	-4.48e-3	3	NC	1	NC	1
533	M5	1	max	.026	3	.371	2	.625	4	0	1	NC	1	NC	1
534			min	-.018	2	-.023	3	0	1	-8.777e-6	4	NC	1	NC	1
535		2	max	.026	3	.183	2	.612	4	7.329e-3	4	NC	5	NC	1
536			min	-.018	2	-.014	3	0	1	0	1	730.944	2	9622.033	4
537		3	max	.026	3	.036	3	.594	4	1.449e-2	4	NC	15	NC	1
538			min	-.018	2	-.03	2	0	1	0	1	340.839	2	5589.864	4
539		4	max	.025	3	.157	3	.575	4	1.181e-2	4	8301.05	15	NC	1
540			min	-.018	2	-.29	2	0	1	0	1	206.451	2	4243.304	4
541		5	max	.025	3	.33	3	.553	4	9.123e-3	4	5766.53	15	NC	1
542			min	-.017	2	-.575	2	0	1	0	1	143.992	2	3566.312	4
543		6	max	.024	3	.526	3	.531	4	6.44e-3	4	4415.686	15	NC	1
544			min	-.017	2	-.861	2	0	1	0	1	110.538	2	3136.657	4
545		7	max	.024	3	.72	3	.509	4	3.757e-3	4	3639.868	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.016	2	-1.121	2	0	1	0	1	91.25	2	2802.801	4
547	8	max	.023	3	.883	3	.487	4	1.073e-3	4	3190.771	15	NC	1
548		min	-.016	2	-1.331	1	0	1	0	1	79.948	1	2497.331	4
549	9	max	.023	3	.989	3	.466	4	3.533e-8	14	2960.934	15	NC	1
550		min	-.016	2	-1.464	1	0	1	-4.636e-6	5	74.105	1	2253.276	4
551	10	max	.022	3	1.027	3	.44	4	1.683e-7	14	2891.694	15	NC	1
552		min	-.016	2	-1.509	1	0	1	-4.397e-6	5	72.373	1	2211.941	4
553	11	max	.022	3	1.002	3	.411	4	3.013e-7	14	2961.073	15	NC	1
554		min	-.015	2	-1.464	1	0	1	-4.157e-6	5	74.239	1	2261.209	4
555	12	max	.021	3	.914	3	.382	4	7.187e-4	4	3191.098	15	NC	1
556		min	-.015	2	-1.327	1	0	1	0	1	80.396	1	2331.506	4
557	13	max	.02	3	.772	3	.346	4	2.516e-3	4	3640.521	15	NC	1
558		min	-.015	2	-1.11	1	0	1	0	1	92.567	1	2737.247	4
559	14	max	.02	3	.594	3	.305	4	4.313e-3	4	4416.94	15	NC	1
560		min	-.014	2	-.84	1	0	1	0	1	113.881	1	3843.036	4
561	15	max	.019	3	.396	3	.261	4	6.111e-3	4	5768.983	15	NC	1
562		min	-.014	2	-.548	1	0	1	0	1	151.701	1	7151.612	5
563	16	max	.019	3	.196	3	.219	4	7.908e-3	4	8306.168	15	NC	1
564		min	-.014	2	-.263	1	0	1	0	1	224.485	1	NC	1
565	17	max	.018	3	.012	3	.182	4	9.705e-3	4	NC	15	NC	1
566		min	-.014	2	-.016	2	0	1	0	1	386.251	1	NC	1
567	18	max	.018	3	.175	1	.153	4	4.91e-3	4	NC	5	NC	1
568		min	-.014	2	-.143	3	0	1	0	1	855.974	1	NC	1
569	19	max	.018	3	.33	1	.133	4	0	1	NC	1	NC	1
570		min	-.014	2	-.28	3	0	1	-4.215e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.211	.625	4	1.702e-2	3	NC	1	NC	1
572		min	-.005	2	-.054	3	0	1	-9.048e-3	1	NC	1	NC	1
573	2	max	.009	3	.104	2	.61	4	8.449e-3	3	NC	5	NC	1
574		min	-.005	2	-.027	3	0	12	-4.367e-3	1	1266.902	2	NC	1
575	3	max	.009	3	.012	3	.593	4	1.443e-2	4	NC	5	NC	1
576		min	-.005	2	-.01	2	0	12	-2.093e-5	10	613.171	2	5993.457	4
577	4	max	.008	3	.073	3	.573	4	1.137e-2	5	NC	15	NC	1
578		min	-.005	2	-.137	2	0	12	-4.215e-3	1	389.855	2	4431.683	4
579	5	max	.008	3	.148	3	.552	4	8.591e-3	5	NC	15	NC	1
580		min	-.005	2	-.268	2	0	12	-8.585e-3	1	282.95	2	3635.504	4
581	6	max	.008	3	.229	3	.531	4	1.174e-2	3	7973.377	15	NC	1
582		min	-.005	2	-.394	2	0	10	-1.295e-2	1	223.8	2	3138.463	4
583	7	max	.008	3	.306	3	.509	4	1.562e-2	3	6739.316	15	NC	1
584		min	-.004	2	-.507	2	0	1	-1.733e-2	1	188.768	2	2777.389	4
585	8	max	.008	3	.37	3	.488	4	1.951e-2	3	6007.978	15	NC	1
586		min	-.004	2	-.596	2	0	1	-2.17e-2	1	168.001	2	2479.716	5
587	9	max	.008	3	.412	3	.466	4	1.999e-2	3	5624.818	15	NC	1
588		min	-.004	2	-.652	2	0	12	-2.384e-2	1	157.014	1	2252.154	4
589	10	max	.007	3	.428	3	.44	4	1.82e-2	3	5507.56	15	NC	1
590		min	-.004	2	-.671	2	0	1	-2.459e-2	2	153.711	1	2192.005	4
591	11	max	.007	3	.418	3	.411	4	1.641e-2	3	5624.542	15	NC	1
592		min	-.004	2	-.652	2	0	1	-2.592e-2	2	157.263	1	2235.7	4
593	12	max	.007	3	.383	3	.381	4	1.42e-2	3	6007.443	15	NC	1
594		min	-.004	2	-.594	2	0	12	-2.475e-2	2	168.795	1	2361.023	4
595	13	max	.007	3	.326	3	.345	4	1.136e-2	3	6738.478	15	NC	1
596		min	-.004	2	-.502	1	0	10	-1.985e-2	2	190.99	1	2811.052	4
597	14	max	.007	3	.254	3	.304	4	8.525e-3	3	7972.096	15	NC	1
598		min	-.004	2	-.386	1	-.002	1	-1.495e-2	2	228.722	1	3876.683	5
599	15	max	.006	3	.172	3	.261	4	5.84e-3	5	NC	15	NC	1
600		min	-.004	2	-.258	1	-.004	1	-1.004e-2	2	293.117	1	6626.801	5
601	16	max	.006	3	.087	3	.22	4	7.805e-3	5	NC	15	NC	1
602		min	-.004	2	-.127	1	-.007	1	-5.142e-3	2	411.011	1	NC	1



Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.006	3	.004	3	.184	4	9.83e-3	4	NC	5	NC	1
604		min	-.004	2	-.006	2	-.007	1	-4.939e-4	1	659.93	1	NC	1
605	18	max	.006	3	.101	1	.155	4	4.776e-3	5	NC	5	NC	1
606		min	-.004	2	-.071	3	-.005	1	-6.46e-3	2	1384.091	1	NC	1
607	19	max	.006	3	.197	1	.133	4	4.48e-3	3	NC	1	NC	1
608		min	-.004	2	-.141	3	0	12	-1.288e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

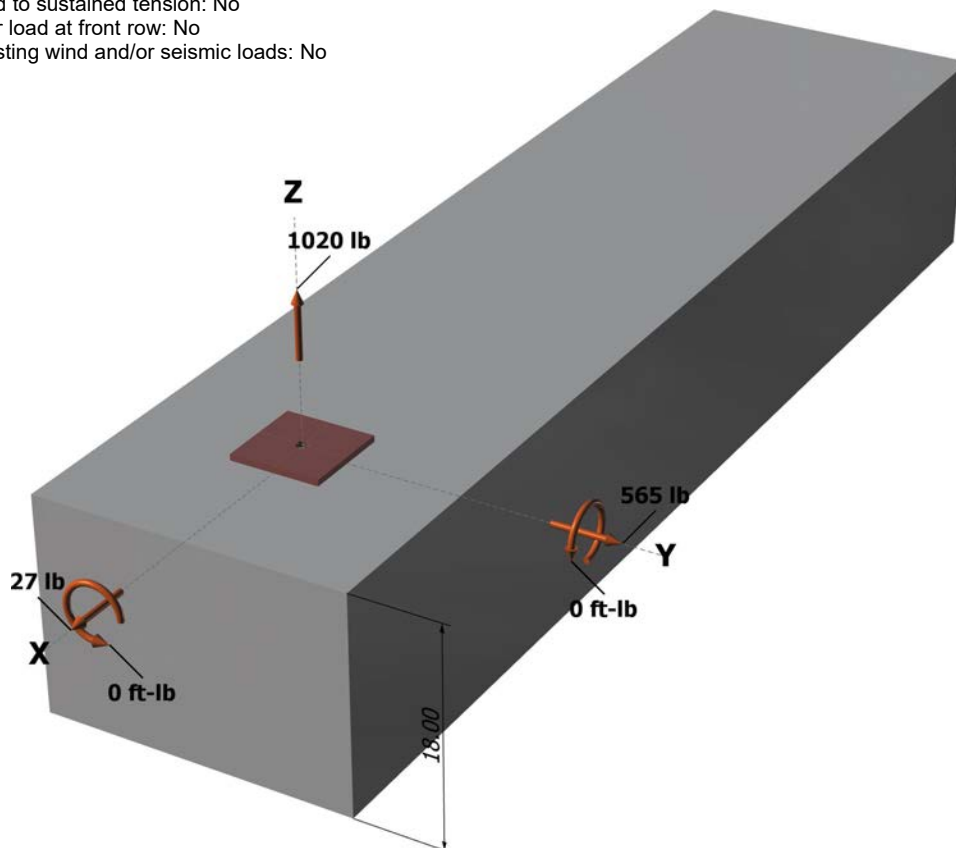
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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# Anchor Designer™ Software Version 2.4.6025.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
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Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

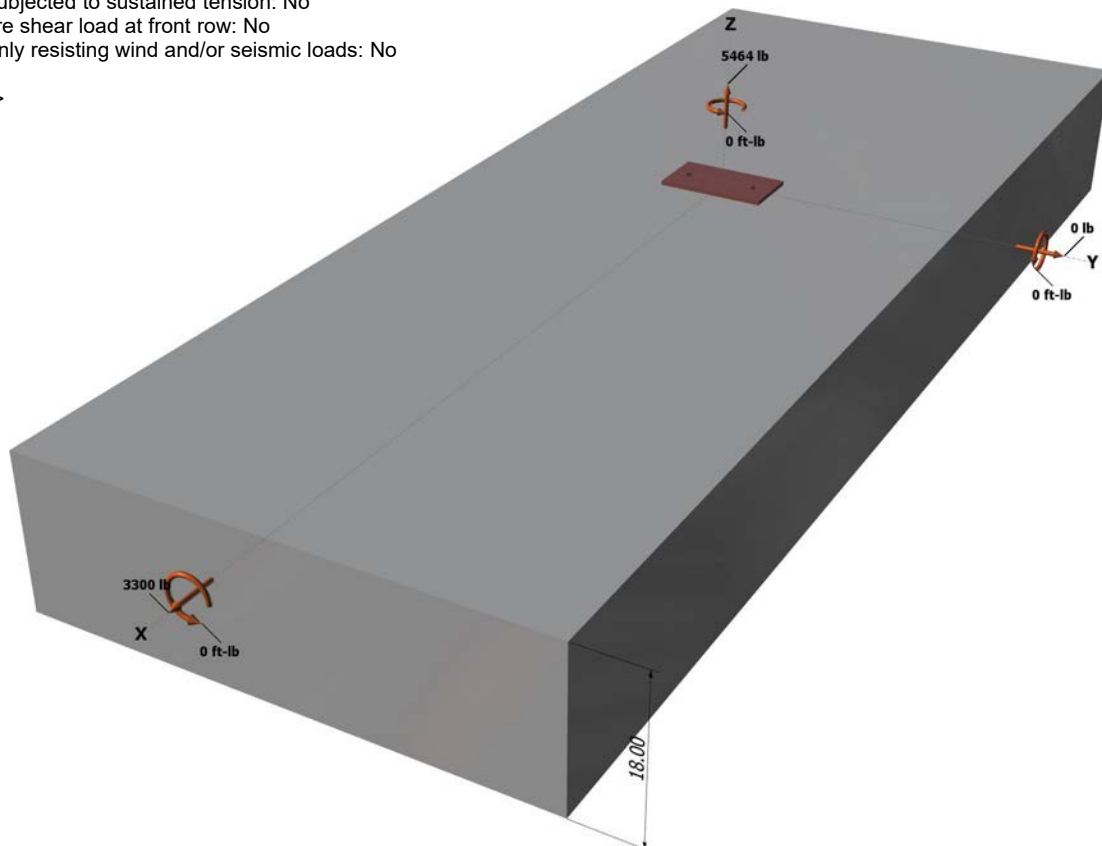
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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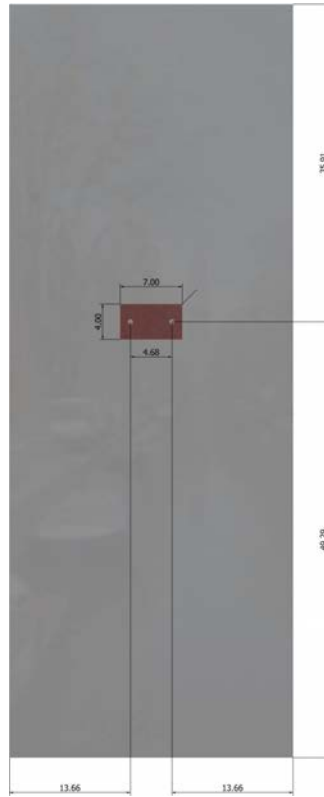




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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

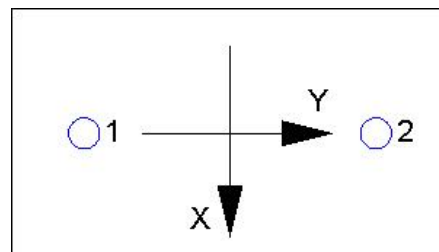
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.