

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	160 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 40.19 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	99 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-2.330 k-ft
$M_z$ =	0.004 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>84%</b>

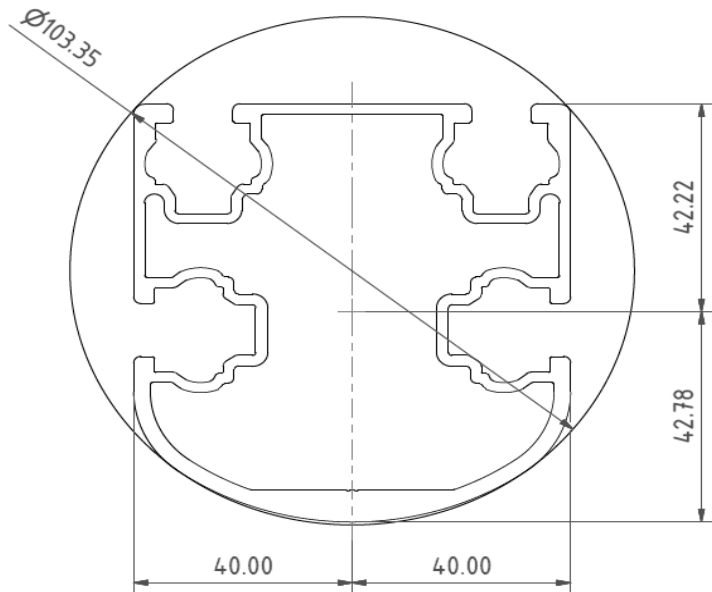


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

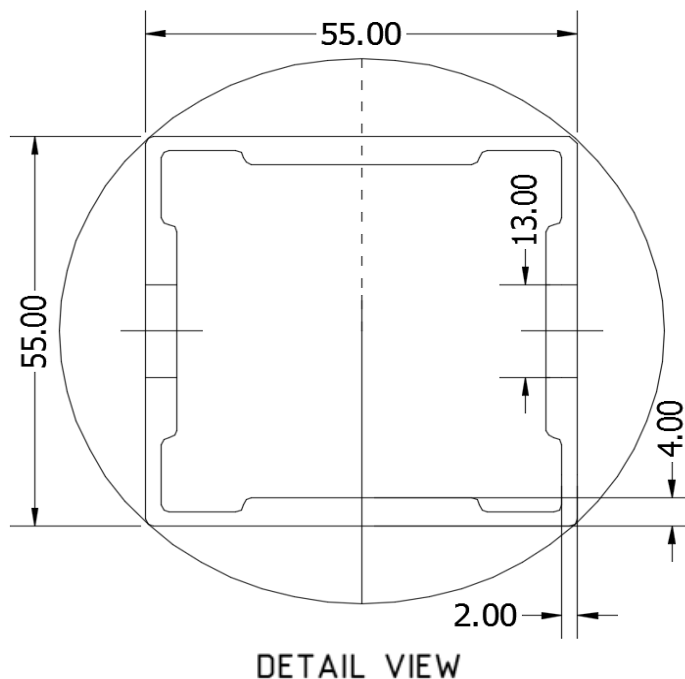
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.271 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.885 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>96%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

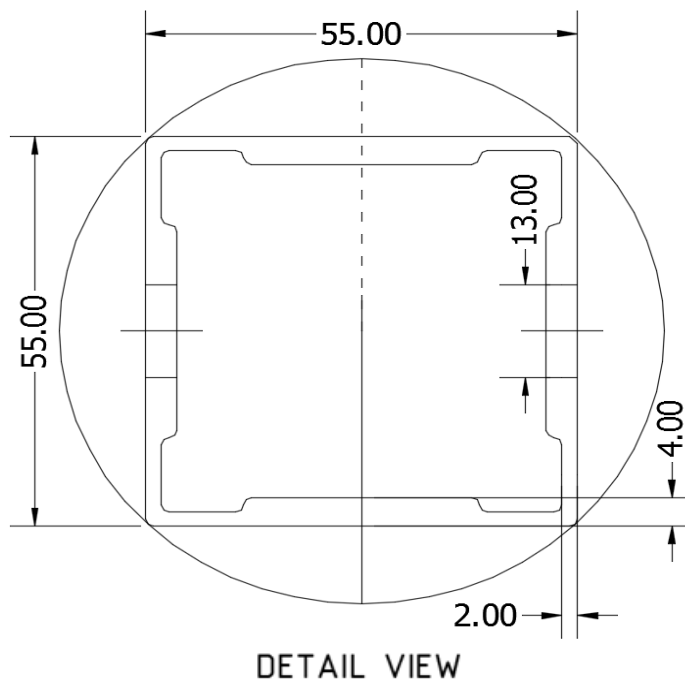
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.447 k-ft
$P_n$ =	0.517 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>34%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.506 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>35%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	63.42 in
$\Phi F_{ty \text{ AXIAL}}$ =	12.77 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.525 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	12.545 k
Utilization =	<u>29%</u>



DETAIL VIEW

### 5. FOUNDATION DESIGN CALCULATIONS

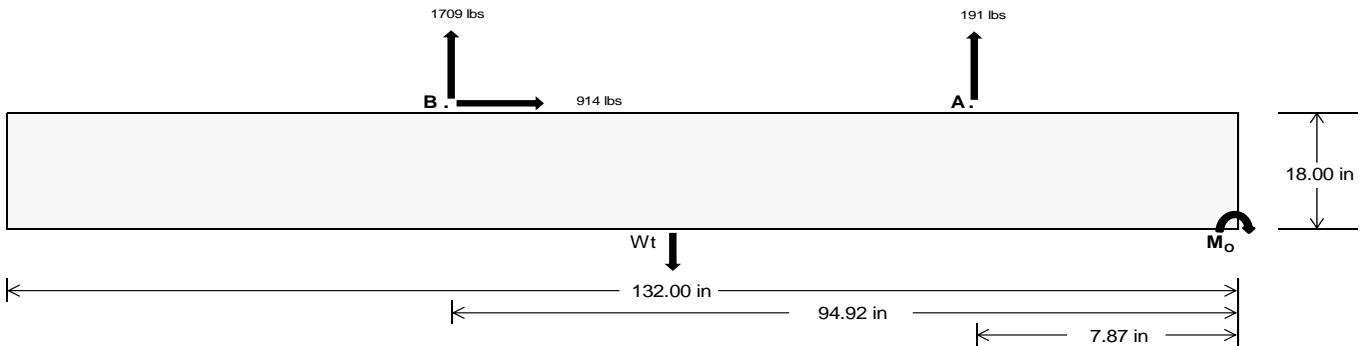
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>848.00</u>	<u>7419.79</u>	k
Compressive Load =	<u>3912.19</u>	<u>5459.55</u>	k
Lateral Load =	<u>307.00</u>	<u>3962.95</u>	k
Moment (Weak Axis) =	<u>0.60</u>	<u>0.25</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 180154.6$  in-lbs  
Resisting Force Required = 2729.61 lbs  
S.F. = 1.67  
Weight Required = 4549.36 lbs  
Minimum Width = 38 in  
Weight Provided = 7576.25 lbs

### Sliding

Force = 914.33 lbs  
Friction = 0.4  
Weight Required = 2285.83 lbs  
Resisting Weight = 7576.25 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 914.33 lbs  
Cohesion = 130 psf  
Area = 34.83 ft<sup>2</sup>  
Resisting = 3788.13 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 132in long x 38in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 38in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 38in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(3.17 \text{ ft}) =$ 

38 in	39 in	40 in	41 in
7576 lbs	7776 lbs	7975 lbs	8174 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in
$F_A$	1145 lbs	1145 lbs	1145 lbs	1145 lbs	1588 lbs	1588 lbs	1588 lbs	1588 lbs	1942 lbs	1942 lbs	1942 lbs	1942 lbs	-383 lbs	-383 lbs	-383 lbs	-383 lbs
$F_B$	1128 lbs	1128 lbs	1128 lbs	1128 lbs	2303 lbs	2303 lbs	2303 lbs	2303 lbs	2468 lbs	2468 lbs	2468 lbs	2468 lbs	-3417 lbs	-3417 lbs	-3417 lbs	-3417 lbs
$F_V$	145 lbs	145 lbs	145 lbs	145 lbs	1632 lbs	1632 lbs	1632 lbs	1632 lbs	1321 lbs	1321 lbs	1321 lbs	1321 lbs	-1829 lbs	-1829 lbs	-1829 lbs	-1829 lbs
$P_{total}$	9850 lbs	10049 lbs	10249 lbs	10448 lbs	11467 lbs	11667 lbs	11866 lbs	12065 lbs	11986 lbs	12185 lbs	12384 lbs	12584 lbs	745 lbs	865 lbs	985 lbs	1104 lbs
$M$	3046 lbs-ft	3046 lbs-ft	3046 lbs-ft	3046 lbs-ft	4587 lbs-ft	4587 lbs-ft	4587 lbs-ft	4587 lbs-ft	5438 lbs-ft	5438 lbs-ft	5438 lbs-ft	5438 lbs-ft	3638 lbs-ft	3638 lbs-ft	3638 lbs-ft	3638 lbs-ft
$e$	0.31 ft	0.30 ft	0.30 ft	0.29 ft	0.40 ft	0.39 ft	0.39 ft	0.38 ft	0.45 ft	0.45 ft	0.44 ft	0.43 ft	4.88 ft	4.21 ft	3.69 ft	3.29 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	235.1 psf	234.6 psf	234.2 psf	233.8 psf	257.4 psf	256.3 psf	255.4 psf	254.5 psf	258.9 psf	257.9 psf	256.9 psf	255.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	330.5 psf	327.6 psf	324.8 psf	322.2 psf	401.0 psf	396.3 psf	391.9 psf	387.6 psf	429.2 psf	423.8 psf	418.7 psf	413.7 psf	253.3 psf	137.1 psf	109.1 psf	97.7 psf

Maximum Bearing Pressure = 429 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 38in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

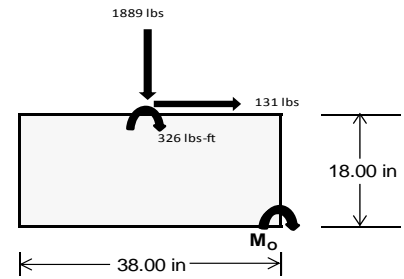
### Overturning Check

$M_o = 2469.1 \text{ ft-lbs}$   
 Resisting Force Required = 1559.45 lbs  
 S.F. = 1.67  
 Weight Required = 2599.08 lbs  
 Minimum Width = **38 in**  
 Weight Provided = 7576.25 lbs

*A minimum 132in long x 38in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	38 in			38 in			38 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	249 lbs	530 lbs	175 lbs	699 lbs	1889 lbs	642 lbs	99 lbs	155 lbs	25 lbs
$F_h$	180 lbs	177 lbs	182 lbs	133 lbs	131 lbs	141 lbs	181 lbs	178 lbs	181 lbs
$P_{total}$	9629 lbs	9910 lbs	9554 lbs	9628 lbs	10818 lbs	9571 lbs	2841 lbs	2898 lbs	2768 lbs
$M$	698 lbs-ft	691 lbs-ft	706 lbs-ft	523 lbs-ft	522 lbs-ft	547 lbs-ft	698 lbs-ft	690 lbs-ft	700 lbs-ft
$e$	0.07 ft	0.07 ft	0.07 ft	0.05 ft	0.05 ft	0.06 ft	0.25 ft	0.24 ft	0.25 ft
$L/6$	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft
$f_{min}$	238.4 psf	246.9 psf	235.9 psf	248.0 psf	282.2 psf	245.0 psf	43.6 psf	45.7 psf	41.4 psf
$f_{max}$	314.4 psf	322.1 psf	312.7 psf	304.8 psf	338.9 psf	304.5 psf	119.5 psf	120.7 psf	117.5 psf



Maximum Bearing Pressure = 339 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 38in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 38in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.950 k
Allowable Uplift =	1.214 k
Utilization =	<u>78%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.808 k
Allowable Uplift =	4.357 k
Utilization =	<u>64%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.009 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>41%</u>

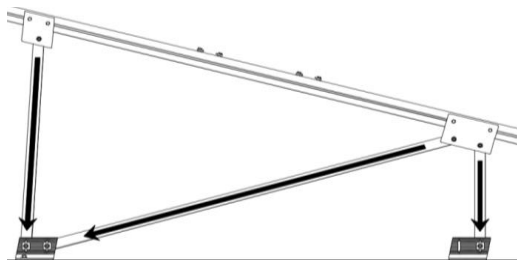
#### Rear Strut

Maximum Axial Load =	5.024 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>68%</u>

#### Diagonal Strut

Maximum Axial Load =	2.631 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>35%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	46.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.938 in
	<u>0.474 ≤ 0.938, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 99 \text{ in}$$

$$J = 0.432$$

$$273.88$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 99$$

$$J = 0.432$$

$$174.171$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \text{ in} \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.46712$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7854$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 12.7711 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 12.77 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 13.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.9	-46.9	0	0
2	M14	Y	-46.9	-46.9	0	0
3	M15	Y	-46.9	-46.9	0	0
4	M16	Y	-46.9	-46.9	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-123.3	-123.3	0	0
2	M14	y	-123.3	-123.3	0	0
3	M15	y	-190.554	-190.554	0	0
4	M16	y	-190.554	-190.554	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	280.227	280.227	0	0
2	M14	y	212.973	212.973	0	0
3	M15	y	112.091	112.091	0	0
4	M16	y	112.091	112.091	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 25° 160mph 30psf 8.25ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	70.871	1	809.591	2	-5.165	12	.015	2	.23	1	1.395	2
20			min	5.125	12	-1418.332	3	-158.917	1	-.001	3	.004	12	-2.33	3
21		11	max	70.871	1	667.245	2	-3.781	12	.015	2	.1	1	.718	2
22			min	5.125	12	-1165.909	3	-125.258	1	0	15	-.002	3	-1.146	3
23		12	max	70.871	1	524.898	2	-2.397	12	.015	2	.045	4	.171	2
24			min	5.125	12	-913.487	3	-91.599	1	0	15	-.006	3	-.193	3
25		13	max	70.871	1	382.551	2	-1.013	12	.015	2	.021	5	.529	3
26			min	5.125	12	-661.064	3	-57.94	1	0	15	-.068	1	-.245	2
27		14	max	70.871	1	240.204	2	.775	3	.015	2	0	15	1.019	3
28			min	4.061	15	-408.641	3	-27.995	4	0	15	-.106	1	-.53	2
29		15	max	70.871	1	97.858	2	9.378	1	.015	2	-.004	12	1.278	3
30			min	-3.234	5	-156.218	3	-20.401	5	0	15	-.113	1	-.685	2
31		16	max	70.871	1	96.205	3	43.037	1	.015	2	-.002	12	1.306	3
32			min	-12.644	5	-44.489	2	-18.259	5	0	15	-.089	1	-.71	2
33		17	max	70.871	1	348.628	3	76.696	1	.015	2	.003	3	1.102	3
34			min	-22.054	5	-186.836	2	-16.118	5	0	15	-.062	4	-.603	2
35		18	max	70.871	1	601.05	3	110.355	1	.015	2	.052	1	.667	3
36			min	-31.463	5	-329.183	2	-13.977	5	0	15	-.068	5	-.367	2
37		19	max	70.871	1	853.473	3	144.014	1	.015	2	.169	1	0	2
38			min	-40.873	5	-471.529	2	-11.836	5	0	15	-.08	5	0	3
39	M14	1	max	44.19	4	521.426	2	-7.512	12	.012	3	.197	1	0	1
40			min	2.282	12	-675.17	3	-149.228	1	-.013	2	.014	12	0	3
41		2	max	37.128	1	379.079	2	-6.128	12	.012	3	.131	4	.531	3
42			min	2.282	12	-484.397	3	-115.569	1	-.013	2	.006	10	-.413	2
43		3	max	37.128	1	236.732	2	-4.744	12	.012	3	.076	5	.888	3
44			min	2.282	12	-293.624	3	-81.91	1	-.013	2	-.015	1	-.695	2
45		4	max	37.128	1	94.386	2	-3.36	12	.012	3	.042	5	1.07	3
46			min	2.282	12	-102.85	3	-48.251	1	-.013	2	-.074	1	-.847	2
47		5	max	37.128	1	87.923	3	-.729	10	.012	3	.01	5	1.077	3
48			min	-1.663	5	-47.961	2	-37.921	4	-.013	2	-.103	1	-.868	2
49		6	max	37.128	1	278.696	3	19.067	1	.012	3	-.005	12	.909	3
50			min	-11.073	5	-190.308	2	-31.888	5	-.013	2	-.101	1	-.759	2
51		7	max	37.128	1	469.47	3	52.726	1	.012	3	-.005	12	.566	3
52			min	-20.483	5	-332.655	2	-29.747	5	-.013	2	-.068	1	-.519	2
53		8	max	37.128	1	660.243	3	86.385	1	.012	3	.002	10	.048	3
54			min	-29.893	5	-475.001	2	-27.605	5	-.013	2	-.077	4	-.149	2
55		9	max	37.128	1	851.016	3	120.044	1	.012	3	.09	1	.352	2
56			min	-39.302	5	-617.348	2	-25.464	5	-.013	2	-.099	5	-.645	3
57		10	max	61.541	4	759.695	2	-4.944	12	.013	2	.216	1	.983	2
58			min	2.282	12	-1041.79	3	-153.703	1	-.012	3	.003	12	-1.512	3
59		11	max	52.131	4	617.348	2	-3.56	12	.013	2	.131	4	.352	2
60			min	2.282	12	-851.016	3	-120.044	1	-.012	3	-.002	3	-.645	3
61		12	max	42.721	4	475.001	2	-2.176	12	.013	2	.074	5	.048	3
62			min	2.282	12	-660.243	3	-86.385	1	-.012	3	-.006	3	-.149	2
63		13	max	37.128	1	332.655	2	-.792	12	.013	2	.04	5	.566	3
64			min	2.282	12	-469.47	3	-52.726	1	-.012	3	-.068	1	-.519	2
65		14	max	37.128	1	190.308	2	1.108	3	.013	2	.007	5	.909	3
66			min	2.282	12	-278.696	3	-38.703	4	-.012	3	-.101	1	-.759	2
67		15	max	37.128	1	47.961	2	14.592	1	.013	2	-.004	12	1.077	3
68			min	2.282	12	-87.923	3	-32.064	5	-.012	3	-.103	1	-.868	2
69		16	max	37.128	1	102.85	3	48.251	1	.013	2	-.001	12	1.07	3
70			min	-3.247	5	-94.386	2	-29.923	5	-.012	3	-.074	1	-.847	2
71		17	max	37.128	1	293.624	3	81.91	1	.013	2	.004	3	.888	3
72			min	-12.656	5	-236.732	2	-27.782	5	-.012	3	-.082	4	-.695	2
73		18	max	37.128	1	484.397	3	115.569	1	.013	2	.076	1	.531	3
74			min	-22.066	5	-379.079	2	-25.64	5	-.012	3	-.102	5	-.413	2
75		19	max	37.128	1	675.17	3	149.228	1	.013	2	.197	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76	M15	min	-31.476	5	-521.426	2	-23.499	5	-.012	3	-.125	5	0	3
77		max	71.334	5	729.555	2	-7.406	12	.014	2	.243	4	0	2
78		min	-38.601	1	-363.536	3	-149.235	1	-.01	3	.013	12	0	3
79		2 max	61.925	5	525.558	2	-6.022	12	.014	2	.167	4	.288	3
80		min	-38.601	1	-265.237	3	-115.576	1	-.01	3	.006	10	-.575	2
81		3 max	52.515	5	321.562	2	-4.638	12	.014	2	.102	5	.486	3
82		min	-38.601	1	-166.939	3	-81.917	1	-.01	3	-.015	1	-.964	2
83		4 max	43.105	5	117.566	2	-3.254	12	.014	2	.058	5	.594	3
84		min	-38.601	1	-68.641	3	-58.462	4	-.01	3	-.074	1	-1.165	2
85		5 max	33.695	5	29.657	3	-.781	10	.014	2	.016	5	.612	3
86		min	-38.601	1	-86.43	2	-48.94	4	-.01	3	-.103	1	-1.179	2
87		6 max	24.285	5	127.955	3	19.06	1	.014	2	-.005	12	.54	3
88		min	-38.601	1	-290.427	2	-42.881	5	-.01	3	-.101	1	-1.006	2
89		7 max	14.876	5	226.253	3	52.719	1	.014	2	-.005	12	.378	3
90		min	-38.601	1	-494.423	2	-40.74	5	-.01	3	-.079	4	-.647	2
91		8 max	5.466	5	324.552	3	86.378	1	.014	2	.002	10	.125	3
92		min	-38.601	1	-698.419	2	-38.599	5	-.01	3	-.102	4	-.1	2
93		9 max	-2.579	15	422.85	3	120.037	1	.014	2	.09	1	.634	2
94		min	-38.601	1	-902.415	2	-36.457	5	-.01	3	-.134	5	-.217	3
95		10 max	-2.898	12	1106.412	2	-5.05	12	.01	3	.242	4	1.555	2
96		min	-38.601	1	-521.148	3	-153.696	1	-.014	2	.004	12	-.65	3
97		11 max	-.82	15	902.415	2	-3.666	12	.01	3	.166	4	.634	2
98		min	-38.601	1	-422.85	3	-120.037	1	-.014	2	-.001	3	-.217	3
99		12 max	-2.898	12	698.419	2	-2.282	12	.01	3	.098	5	.125	3
100		min	-38.601	1	-324.552	3	-86.378	1	-.014	2	-.005	3	-.1	2
101		13 max	-2.898	12	494.423	2	-.898	12	.01	3	.054	5	.378	3
102		min	-38.601	1	-226.253	3	-59.265	4	-.014	2	-.068	1	-.647	2
103		14 max	-2.898	12	290.427	2	.932	3	.01	3	.012	5	.54	3
104		min	-38.719	4	-127.955	3	-49.743	4	-.014	2	-.101	1	-1.006	2
105		15 max	-2.898	12	86.43	2	14.599	1	.01	3	-.004	12	.612	3
106		min	-48.129	4	-29.657	3	-43.06	5	-.014	2	-.103	1	-1.179	2
107		16 max	-2.898	12	68.641	3	48.258	1	.01	3	-.001	12	.594	3
108		min	-57.539	4	-117.566	2	-40.919	5	-.014	2	-.084	4	-1.165	2
109		17 max	-2.898	12	166.939	3	81.917	1	.01	3	.004	3	.486	3
110		min	-66.948	4	-321.562	2	-38.778	5	-.014	2	-.108	4	-.964	2
111		18 max	-2.898	12	265.237	3	115.576	1	.01	3	.076	1	.288	3
112		min	-76.358	4	-525.558	2	-36.637	5	-.014	2	-.138	5	-.575	2
113		19 max	-2.898	12	363.536	3	149.235	1	.01	3	.197	1	0	2
114		min	-85.768	4	-729.555	2	-34.495	5	-.014	2	-.171	5	0	5
115	M16	1 max	69.508	5	681.6	2	-6.936	12	.011	2	.186	4	0	2
116		min	-76.019	1	-323.439	3	-144.343	1	-.013	3	.011	12	0	3
117		2 max	60.098	5	477.603	2	-5.552	12	.011	2	.123	4	.251	3
118		min	-76.019	1	-225.141	3	-110.684	1	-.013	3	.004	10	-.531	2
119		3 max	50.689	5	273.607	2	-4.168	12	.011	2	.076	5	.413	3
120		min	-76.019	1	-126.843	3	-77.025	1	-.013	3	-.033	1	-.876	2
121		4 max	41.279	5	69.611	2	-2.784	12	.011	2	.043	5	.484	3
122		min	-76.019	1	-28.545	3	-44.333	4	-.013	3	-.088	1	-1.033	2
123		5 max	31.869	5	69.753	3	-.383	10	.011	2	.013	5	.465	3
124		min	-76.019	1	-134.385	2	-34.811	4	-.013	3	-.112	1	-1.003	2
125		6 max	22.459	5	168.051	3	23.951	1	.011	2	-.005	12	.356	3
126		min	-76.019	1	-338.382	2	-30.003	5	-.013	3	-.106	1	-.787	2
127		7 max	13.05	5	266.35	3	57.61	1	.011	2	-.004	12	.157	3
128		min	-76.019	1	-542.378	2	-27.862	5	-.013	3	-.068	1	-.383	2
129		8 max	3.64	5	364.648	3	91.269	1	.011	2	.003	2	.208	2
130		min	-76.019	1	-746.374	2	-25.72	5	-.013	3	-.068	4	-.132	3
131		9 max	-3.792	15	462.946	3	124.928	1	.011	2	.099	1	.986	2
132		min	-76.019	1	-950.37	2	-23.579	5	-.013	3	-.089	5	-.512	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-4.935	12	1154.367	2	-5.52	12	.013	3	.229	1	1.95	2
134		min	-76.019	1	-561.244	3	-158.587	1	-.011	2	.005	12	-.981	3
135	11	max	-2.574	15	950.37	2	-4.136	12	.013	3	.124	4	.986	2
136		min	-76.019	1	-462.946	3	-124.928	1	-.011	2	0	3	-.512	3
137	12	max	-4.935	12	746.374	2	-2.752	12	.013	3	.067	4	.208	2
138		min	-76.019	1	-364.648	3	-91.269	1	-.011	2	-.004	3	-.132	3
139	13	max	-4.935	12	542.378	2	-1.368	12	.013	3	.033	5	.157	3
140		min	-76.019	1	-266.35	3	-57.61	1	-.011	2	-.068	1	-.383	2
141	14	max	-4.935	12	338.382	2	.184	3	.013	3	.002	5	.356	3
142		min	-76.019	1	-168.051	3	-38.469	4	-.011	2	-.106	1	-.787	2
143	15	max	-4.935	12	134.385	2	9.708	1	.013	3	-.004	12	.465	3
144		min	-76.019	1	-69.753	3	-30.83	5	-.011	2	-.112	1	-1.003	2
145	16	max	-4.935	12	28.545	3	43.367	1	.013	3	-.002	12	.484	3
146		min	-76.019	1	-69.611	2	-28.689	5	-.011	2	-.088	1	-1.033	2
147	17	max	-4.935	12	126.843	3	77.025	1	.013	3	.001	3	.413	3
148		min	-78.055	4	-273.607	2	-26.548	5	-.011	2	-.087	4	-.876	2
149	18	max	-4.935	12	225.141	3	110.684	1	.013	3	.053	1	.251	3
150		min	-87.465	4	-477.603	2	-24.406	5	-.011	2	-.103	5	-.531	2
151	19	max	-4.935	12	323.439	3	144.343	1	.013	3	.17	1	0	2
152		min	-96.875	4	-681.6	2	-22.265	5	-.011	2	-.124	5	0	5
153	M2	1	max	1116.177	2	1.962	4	.458	1	0	0	3	0	1
154		min	-1562.247	3	.478	15	-33.935	4	0	4	0	2	0	1
155	2	max	1116.606	2	1.905	4	.458	1	0	3	0	1	0	15
156		min	-1561.925	3	.465	15	-34.309	4	0	4	-.01	4	0	4
157	3	max	1117.034	2	1.849	4	.458	1	0	3	0	1	0	15
158		min	-1561.604	3	.452	15	-34.682	4	0	4	-.02	4	-.001	4
159	4	max	1117.463	2	1.792	4	.458	1	0	3	0	1	0	15
160		min	-1561.283	3	.438	15	-35.055	4	0	4	-.03	4	-.002	4
161	5	max	1117.891	2	1.735	4	.458	1	0	3	0	1	0	15
162		min	-1560.961	3	.424	12	-35.429	4	0	4	-.04	4	-.002	4
163	6	max	1118.32	2	1.678	4	.458	1	0	3	0	1	0	15
164		min	-1560.64	3	.402	12	-35.802	4	0	4	-.051	4	-.003	4
165	7	max	1118.748	2	1.621	4	.458	1	0	3	0	1	0	15
166		min	-1560.319	3	.38	12	-36.175	4	0	4	-.061	4	-.003	4
167	8	max	1119.177	2	1.565	4	.458	1	0	3	0	1	0	15
168		min	-1559.997	3	.358	12	-36.549	4	0	4	-.072	4	-.004	4
169	9	max	1119.605	2	1.508	4	.458	1	0	3	.001	1	0	12
170		min	-1559.676	3	.336	12	-36.922	4	0	4	-.082	4	-.004	4
171	10	max	1120.034	2	1.451	4	.458	1	0	3	.001	1	-.001	12
172		min	-1559.355	3	.314	12	-37.295	4	0	4	-.093	4	-.004	4
173	11	max	1120.462	2	1.394	4	.458	1	0	3	.001	1	-.001	12
174		min	-1559.033	3	.292	12	-37.668	4	0	4	-.104	4	-.005	4
175	12	max	1120.891	2	1.337	4	.458	1	0	3	.001	1	-.001	12
176		min	-1558.712	3	.269	12	-38.042	4	0	4	-.115	4	-.005	4
177	13	max	1121.319	2	1.281	4	.458	1	0	3	.002	1	-.001	12
178		min	-1558.39	3	.247	12	-38.415	4	0	4	-.126	4	-.006	4
179	14	max	1121.748	2	1.224	4	.458	1	0	3	.002	1	-.001	12
180		min	-1558.069	3	.225	12	-38.788	4	0	4	-.137	4	-.006	4
181	15	max	1122.176	2	1.171	2	.458	1	0	3	.002	1	-.001	12
182		min	-1557.748	3	.203	12	-39.162	4	0	4	-.149	4	-.006	4
183	16	max	1122.605	2	1.127	2	.458	1	0	3	.002	1	-.002	12
184		min	-1557.426	3	.181	12	-39.535	4	0	4	-.16	4	-.007	4
185	17	max	1123.033	2	1.083	2	.458	1	0	3	.002	1	-.002	12
186		min	-1557.105	3	.159	12	-39.908	4	0	4	-.172	4	-.007	4
187	18	max	1123.462	2	1.038	2	.458	1	0	3	.002	1	-.002	12
188		min	-1556.784	3	.137	12	-40.282	4	0	4	-.183	4	-.007	4
189	19	max	1123.89	2	.994	2	.458	1	0	3	.002	1	-.002	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1556.462	3	.115	12	-40.655	4	0	4	-.195	4	-.008	4
191	M3	1	max	717.688	2	7.91	4	2.919	4	0	3	0	.008	4
192		min	-847.84	3	1.871	15	.008	12	0	4	-.021	4	.002	12
193		2	max	717.517	2	7.143	4	3.458	4	0	3	0	.005	2
194		min	-847.968	3	1.691	15	.008	12	0	4	-.019	4	0	12
195		3	max	717.347	2	6.376	4	3.997	4	0	3	0	.002	2
196		min	-848.095	3	1.511	15	.008	12	0	4	-.018	4	-.001	3
197		4	max	717.176	2	5.609	4	4.535	4	0	3	0	0	2
198		min	-848.223	3	1.33	15	.008	12	0	4	-.016	4	-.002	3
199		5	max	717.006	2	4.842	4	5.074	4	0	3	0	0	15
200		min	-848.351	3	1.15	15	.008	12	0	4	-.014	4	-.004	3
201		6	max	716.836	2	4.074	4	5.613	4	0	3	0	1	15
202		min	-848.479	3	.97	15	.008	12	0	4	-.012	5	-.005	6
203		7	max	716.665	2	3.307	4	6.152	4	0	3	0	1	15
204		min	-848.606	3	.789	15	.008	12	0	4	-.01	5	-.007	6
205		8	max	716.495	2	2.54	4	6.69	4	0	3	0	1	15
206		min	-848.734	3	.609	15	.008	12	0	4	-.007	5	-.008	6
207		9	max	716.325	2	1.773	4	7.229	4	0	3	0	1	15
208		min	-848.862	3	.429	15	.008	12	0	4	-.004	5	-.009	6
209		10	max	716.154	2	1.005	4	7.768	4	0	3	0	1	15
210		min	-848.99	3	.217	12	.008	12	0	4	0	5	-.009	6
211		11	max	715.984	2	.367	2	8.307	4	0	3	.003	4	15
212		min	-849.117	3	-.15	3	.008	12	0	4	0	12	-.009	6
213		12	max	715.814	2	-.112	15	8.845	4	0	3	.006	4	15
214		min	-849.245	3	-.598	3	.008	12	0	4	0	12	-.009	6
215		13	max	715.643	2	-.293	15	9.384	4	0	3	.01	4	15
216		min	-849.373	3	-1.297	6	.008	12	0	4	0	12	-.009	6
217		14	max	715.473	2	-.473	15	9.923	4	0	3	.014	4	15
218		min	-849.501	3	-2.064	6	.008	12	0	4	0	12	-.008	6
219		15	max	715.303	2	-.653	15	10.462	4	0	3	.018	4	15
220		min	-849.628	3	-2.832	6	.008	12	0	4	0	12	-.007	6
221		16	max	715.132	2	-.834	15	11	4	0	3	.023	4	15
222		min	-849.756	3	-3.599	6	.008	12	0	4	0	12	-.006	6
223		17	max	714.962	2	-1.014	15	11.539	4	0	3	.028	4	15
224		min	-849.884	3	-4.366	6	.008	12	0	4	0	12	-.004	6
225		18	max	714.792	2	-1.194	15	12.078	4	0	3	.033	4	15
226		min	-850.012	3	-5.133	6	.008	12	0	4	0	12	-.002	6
227		19	max	714.621	2	-1.375	15	12.617	4	0	3	.038	4	1
228		min	-850.14	3	-5.901	6	.008	12	0	4	0	12	0	1
229	M4	1	max	1049.665	1	0	1	-.466	12	0	1	.027	4	1
230		min	-179.306	3	0	1	-234.375	4	0	1	0	12	0	1
231		2	max	1049.835	1	0	1	-.466	12	0	1	0	5	1
232		min	-179.178	3	0	1	-234.522	4	0	1	0	2	0	1
233		3	max	1050.005	1	0	1	-.466	12	0	1	0	12	1
234		min	-179.051	3	0	1	-234.67	4	0	1	-.027	4	0	1
235		4	max	1050.176	1	0	1	-.466	12	0	1	0	12	1
236		min	-178.923	3	0	1	-234.817	4	0	1	-.054	4	0	1
237		5	max	1050.346	1	0	1	-.466	12	0	1	0	12	1
238		min	-178.795	3	0	1	-234.965	4	0	1	-.08	4	0	1
239		6	max	1050.516	1	0	1	-.466	12	0	1	0	12	1
240		min	-178.667	3	0	1	-235.113	4	0	1	-.107	4	0	1
241		7	max	1050.687	1	0	1	-.466	12	0	1	0	12	1
242		min	-178.54	3	0	1	-235.26	4	0	1	-.134	4	0	1
243		8	max	1050.857	1	0	1	-.466	12	0	1	0	12	1
244		min	-178.412	3	0	1	-235.408	4	0	1	-.162	4	0	1
245		9	max	1051.027	1	0	1	-.466	12	0	1	0	12	1
246		min	-178.284	3	0	1	-235.556	4	0	1	-.189	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1051.198	1	0	1	-.466	12	0	1	0	12	0	1
248		min	-178.156	3	0	1	-235.703	4	0	1	-.216	4	0	1
249	11	max	1051.368	1	0	1	-.466	12	0	1	0	12	0	1
250		min	-178.029	3	0	1	-235.851	4	0	1	-.243	4	0	1
251	12	max	1051.538	1	0	1	-.466	12	0	1	0	12	0	1
252		min	-177.901	3	0	1	-235.999	4	0	1	-.27	4	0	1
253	13	max	1051.709	1	0	1	-.466	12	0	1	0	12	0	1
254		min	-177.773	3	0	1	-236.146	4	0	1	-.297	4	0	1
255	14	max	1051.879	1	0	1	-.466	12	0	1	0	12	0	1
256		min	-177.645	3	0	1	-236.294	4	0	1	-.324	4	0	1
257	15	max	1052.049	1	0	1	-.466	12	0	1	0	12	0	1
258		min	-177.517	3	0	1	-236.441	4	0	1	-.351	4	0	1
259	16	max	1052.22	1	0	1	-.466	12	0	1	0	12	0	1
260		min	-177.39	3	0	1	-236.589	4	0	1	-.378	4	0	1
261	17	max	1052.39	1	0	1	-.466	12	0	1	0	12	0	1
262		min	-177.262	3	0	1	-236.737	4	0	1	-.405	4	0	1
263	18	max	1052.56	1	0	1	-.466	12	0	1	0	12	0	1
264		min	-177.134	3	0	1	-236.884	4	0	1	-.433	4	0	1
265	19	max	1052.731	1	0	1	-.466	12	0	1	0	12	0	1
266		min	-177.006	3	0	1	-237.032	4	0	1	-.46	4	0	1
267	M6	1	max	3517.304	2	2.486	2	0	1	0	0	4	0	1
268		min	-5023.693	3	-.121	3	-34.266	4	0	4	0	1	0	1
269	2	max	3517.732	2	2.442	2	0	1	0	1	0	1	0	3
270		min	-5023.372	3	-.154	3	-34.64	4	0	4	-.01	4	0	2
271	3	max	3518.161	2	2.397	2	0	1	0	1	0	1	0	3
272		min	-5023.05	3	-.187	3	-35.013	4	0	4	-.02	4	-.001	2
273	4	max	3518.589	2	2.353	2	0	1	0	1	0	1	0	3
274		min	-5022.729	3	-.22	3	-35.386	4	0	4	-.03	4	-.002	2
275	5	max	3519.018	2	2.309	2	0	1	0	1	0	1	0	3
276		min	-5022.408	3	-.253	3	-35.76	4	0	4	-.041	4	-.003	2
277	6	max	3519.446	2	2.265	2	0	1	0	1	0	1	0	3
278		min	-5022.086	3	-.286	3	-36.133	4	0	4	-.051	4	-.003	2
279	7	max	3519.875	2	2.22	2	0	1	0	1	0	1	0	3
280		min	-5021.765	3	-.32	3	-36.506	4	0	4	-.062	4	-.004	2
281	8	max	3520.303	2	2.176	2	0	1	0	1	0	1	0	3
282		min	-5021.443	3	-.353	3	-36.88	4	0	4	-.072	4	-.005	2
283	9	max	3520.732	2	2.132	2	0	1	0	1	0	1	0	3
284		min	-5021.122	3	-.386	3	-37.253	4	0	4	-.083	4	-.005	2
285	10	max	3521.16	2	2.088	2	0	1	0	1	0	1	0	3
286		min	-5020.801	3	-.419	3	-37.626	4	0	4	-.094	4	-.006	2
287	11	max	3521.589	2	2.043	2	0	1	0	1	0	1	0	3
288		min	-5020.479	3	-.452	3	-38	4	0	4	-.105	4	-.007	2
289	12	max	3522.017	2	1.999	2	0	1	0	1	0	1	0	3
290		min	-5020.158	3	-.486	3	-38.373	4	0	4	-.116	4	-.007	2
291	13	max	3522.446	2	1.955	2	0	1	0	1	0	1	.001	3
292		min	-5019.837	3	-.519	3	-38.746	4	0	4	-.127	4	-.008	2
293	14	max	3522.874	2	1.911	2	0	1	0	1	0	1	.001	3
294		min	-5019.515	3	-.552	3	-39.12	4	0	4	-.139	4	-.008	2
295	15	max	3523.303	2	1.866	2	0	1	0	1	0	1	.001	3
296		min	-5019.194	3	-.585	3	-39.493	4	0	4	-.15	4	-.009	2
297	16	max	3523.731	2	1.822	2	0	1	0	1	0	1	.002	3
298		min	-5018.873	3	-.618	3	-39.866	4	0	4	-.161	4	-.009	2
299	17	max	3524.16	2	1.778	2	0	1	0	1	0	1	.002	3
300		min	-5018.551	3	-.652	3	-40.24	4	0	4	-.173	4	-.01	2
301	18	max	3524.588	2	1.734	2	0	1	0	1	0	1	.002	3
302		min	-5018.23	3	-.685	3	-40.613	4	0	4	-.185	4	-.01	2
303	19	max	3525.017	2	1.689	2	0	1	0	1	0	1	.002	3



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Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-5017.908	3	-.718	3	-40.986	4	0	4	-.197	4	-.011	2
305	M7	1	max	2506.155	2	7.911	6	2.729	4	0	1	0	1	.011	2
306			min	-2628.582	3	1.858	15	0	1	0	4	-.021	4	-.002	3
307		2	max	2505.984	2	7.144	6	3.268	4	0	1	0	1	.008	2
308			min	-2628.709	3	1.677	15	0	1	0	4	-.02	4	-.004	3
309		3	max	2505.814	2	6.377	6	3.807	4	0	1	0	1	.006	2
310			min	-2628.837	3	1.497	15	0	1	0	4	-.018	4	-.005	3
311		4	max	2505.644	2	5.61	6	4.345	4	0	1	0	1	.003	2
312			min	-2628.965	3	1.317	15	0	1	0	4	-.017	4	-.006	3
313		5	max	2505.473	2	4.843	6	4.884	4	0	1	0	1	.001	2
314			min	-2629.093	3	1.136	15	0	1	0	4	-.015	4	-.007	3
315		6	max	2505.303	2	4.075	6	5.423	4	0	1	0	1	0	2
316			min	-2629.22	3	.956	15	0	1	0	4	-.012	4	-.008	3
317		7	max	2505.133	2	3.308	6	5.961	4	0	1	0	1	-.002	15
318			min	-2629.348	3	.761	12	0	1	0	4	-.01	4	-.008	3
319		8	max	2504.962	2	2.642	2	6.5	4	0	1	0	1	-.002	15
320			min	-2629.476	3	.462	12	0	1	0	4	-.007	4	-.009	3
321		9	max	2504.792	2	2.044	2	7.039	4	0	1	0	1	-.002	15
322			min	-2629.604	3	.158	3	0	1	0	4	-.005	5	-.009	3
323		10	max	2504.621	2	1.447	2	7.578	4	0	1	0	1	-.002	15
324			min	-2629.731	3	-.29	3	0	1	0	4	-.002	5	-.009	4
325		11	max	2504.451	2	.849	2	8.116	4	0	1	.002	4	-.002	15
326			min	-2629.859	3	-.739	3	0	1	0	4	0	1	-.009	4
327		12	max	2504.281	2	.251	2	8.655	4	0	1	.005	4	-.002	15
328			min	-2629.987	3	-1.187	3	0	1	0	4	0	1	-.009	4
329		13	max	2504.11	2	-.307	15	9.194	4	0	1	.009	4	-.002	15
330			min	-2630.115	3	-1.636	3	0	1	0	4	0	1	-.009	4
331		14	max	2503.94	2	-.487	15	9.733	4	0	1	.013	4	-.002	15
332			min	-2630.242	3	-2.084	3	0	1	0	4	0	1	-.008	4
333		15	max	2503.77	2	-.667	15	10.271	4	0	1	.017	4	-.002	15
334			min	-2630.37	3	-2.83	4	0	1	0	4	0	1	-.007	4
335		16	max	2503.599	2	-.848	15	10.81	4	0	1	.022	4	-.001	15
336			min	-2630.498	3	-3.597	4	0	1	0	4	0	1	-.006	4
337		17	max	2503.429	2	-1.028	15	11.349	4	0	1	.026	4	-.001	15
338			min	-2630.626	3	-4.364	4	0	1	0	4	0	1	-.004	4
339		18	max	2503.259	2	-1.208	15	11.888	4	0	1	.031	4	0	15
340			min	-2630.753	3	-5.131	4	0	1	0	4	0	1	-.002	4
341		19	max	2503.088	2	-1.389	15	12.426	4	0	1	.036	4	0	1
342			min	-2630.881	3	-5.899	4	0	1	0	4	0	1	0	1
343	M8	1	max	3006.308	2	0	1	0	1	0	1	.026	4	0	1
344			min	-654.608	3	0	1	-227.648	4	0	1	0	1	0	1
345		2	max	3006.478	2	0	1	0	1	0	1	0	5	0	1
346			min	-654.481	3	0	1	-227.796	4	0	1	0	14	0	1
347		3	max	3006.649	2	0	1	0	1	0	1	0	1	0	1
348			min	-654.353	3	0	1	-227.943	4	0	1	-.026	4	0	1
349		4	max	3006.819	2	0	1	0	1	0	1	0	1	0	1
350			min	-654.225	3	0	1	-228.091	4	0	1	-.052	4	0	1
351		5	max	3006.989	2	0	1	0	1	0	1	0	1	0	1
352			min	-654.097	3	0	1	-228.239	4	0	1	-.079	4	0	1
353		6	max	3007.16	2	0	1	0	1	0	1	0	1	0	1
354			min	-653.97	3	0	1	-228.386	4	0	1	-.105	4	0	1
355		7	max	3007.33	2	0	1	0	1	0	1	0	1	0	1
356			min	-653.842	3	0	1	-228.534	4	0	1	-.131	4	0	1
357		8	max	3007.5	2	0	1	0	1	0	1	0	1	0	1
358			min	-653.714	3	0	1	-228.681	4	0	1	-.157	4	0	1
359		9	max	3007.671	2	0	1	0	1	0	1	0	1	0	1
360			min	-653.586	3	0	1	-228.829	4	0	1	-.184	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3007.841	2	0	1	0	1	0	1	0	1	0	1
362			min	-653.459	3	0	1	-228.977	4	0	1	-.21	4	0	1
363		11	max	3008.011	2	0	1	0	1	0	1	0	1	0	1
364			min	-653.331	3	0	1	-229.124	4	0	1	-.236	4	0	1
365		12	max	3008.182	2	0	1	0	1	0	1	0	1	0	1
366			min	-653.203	3	0	1	-229.272	4	0	1	-.262	4	0	1
367		13	max	3008.352	2	0	1	0	1	0	1	0	1	0	1
368			min	-653.075	3	0	1	-229.42	4	0	1	-.289	4	0	1
369		14	max	3008.522	2	0	1	0	1	0	1	0	1	0	1
370			min	-652.948	3	0	1	-229.567	4	0	1	-.315	4	0	1
371		15	max	3008.693	2	0	1	0	1	0	1	0	1	0	1
372			min	-652.82	3	0	1	-229.715	4	0	1	-.342	4	0	1
373		16	max	3008.863	2	0	1	0	1	0	1	0	1	0	1
374			min	-652.692	3	0	1	-229.863	4	0	1	-.368	4	0	1
375		17	max	3009.033	2	0	1	0	1	0	1	0	1	0	1
376			min	-652.564	3	0	1	-230.01	4	0	1	-.394	4	0	1
377		18	max	3009.204	2	0	1	0	1	0	1	0	1	0	1
378			min	-652.436	3	0	1	-230.158	4	0	1	-.421	4	0	1
379		19	max	3009.374	2	0	1	0	1	0	1	0	1	0	1
380			min	-652.309	3	0	1	-230.305	4	0	1	-.447	4	0	1
381	M10	1	max	1116.177	2	1.886	6	-.03	12	0	1	0	2	0	1
382			min	-1562.247	3	.427	15	-34.199	4	0	5	0	3	0	1
383		2	max	1116.606	2	1.829	6	-.03	12	0	1	0	10	0	15
384			min	-1561.925	3	.414	15	-34.572	4	0	5	-.01	4	0	6
385		3	max	1117.034	2	1.772	6	-.03	12	0	1	0	10	0	15
386			min	-1561.604	3	.4	15	-34.945	4	0	5	-.02	4	-.001	6
387		4	max	1117.463	2	1.716	6	-.03	12	0	1	0	10	0	15
388			min	-1561.283	3	.387	15	-35.319	4	0	5	-.03	4	-.002	6
389		5	max	1117.891	2	1.659	6	-.03	12	0	1	0	10	0	15
390			min	-1560.961	3	.374	15	-35.692	4	0	5	-.041	4	-.002	6
391		6	max	1118.32	2	1.602	6	-.03	12	0	1	0	12	0	15
392			min	-1560.64	3	.36	15	-36.065	4	0	5	-.051	4	-.003	6
393		7	max	1118.748	2	1.545	6	-.03	12	0	1	0	12	0	15
394			min	-1560.319	3	.347	15	-36.439	4	0	5	-.062	4	-.003	6
395		8	max	1119.177	2	1.488	6	-.03	12	0	1	0	12	0	15
396			min	-1559.997	3	.333	15	-36.812	4	0	5	-.072	4	-.003	6
397		9	max	1119.605	2	1.437	2	-.03	12	0	1	0	12	0	15
398			min	-1559.676	3	.32	15	-37.185	4	0	5	-.083	4	-.004	6
399		10	max	1120.034	2	1.392	2	-.03	12	0	1	0	12	0	15
400			min	-1559.355	3	.307	15	-37.559	4	0	5	-.094	4	-.004	6
401		11	max	1120.462	2	1.348	2	-.03	12	0	1	0	12	-.001	15
402			min	-1559.033	3	.292	12	-37.932	4	0	5	-.105	4	-.005	6
403		12	max	1120.891	2	1.304	2	-.03	12	0	1	0	12	-.001	15
404			min	-1558.712	3	.269	12	-38.305	4	0	5	-.116	4	-.005	6
405		13	max	1121.319	2	1.26	2	-.03	12	0	1	0	12	-.001	15
406			min	-1558.39	3	.247	12	-38.679	4	0	5	-.127	4	-.005	6
407		14	max	1121.748	2	1.215	2	-.03	12	0	1	0	12	-.001	15
408			min	-1558.069	3	.225	12	-39.052	4	0	5	-.138	4	-.006	6
409		15	max	1122.176	2	1.171	2	-.03	12	0	1	0	12	-.001	15
410			min	-1557.748	3	.203	12	-39.425	4	0	5	-.15	4	-.006	6
411		16	max	1122.605	2	1.127	2	-.03	12	0	1	0	12	-.001	15
412			min	-1557.426	3	.181	12	-39.799	4	0	5	-.161	4	-.006	6
413		17	max	1123.033	2	1.083	2	-.03	12	0	1	0	12	-.001	15
414			min	-1557.105	3	.159	12	-40.172	4	0	5	-.173	4	-.007	2
415		18	max	1123.462	2	1.038	2	-.03	12	0	1	0	12	-.002	15
416			min	-1556.784	3	.137	12	-40.545	4	0	5	-.184	4	-.007	2
417		19	max	1123.89	2	.994	2	-.03	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1556.462	3	.115	12	-40.919	4	0	5	-.196	4	-.007	2
419	M11	1	max	717.688	2	7.858	6	2.85	4	0	1	0	12	.007	2
420			min	-847.84	3	1.836	15	-.12	1	0	4	-.021	4	.002	15
421		2	max	717.517	2	7.09	6	3.389	4	0	1	0	12	.005	2
422			min	-847.968	3	1.655	15	-.12	1	0	4	-.02	4	0	12
423		3	max	717.347	2	6.323	6	3.928	4	0	1	0	12	.002	2
424			min	-848.095	3	1.475	15	-.12	1	0	4	-.018	4	-.001	3
425		4	max	717.176	2	5.556	6	4.466	4	0	1	0	12	0	2
426			min	-848.223	3	1.295	15	-.12	1	0	4	-.016	4	-.002	3
427		5	max	717.006	2	4.789	6	5.005	4	0	1	0	12	0	15
428			min	-848.351	3	1.114	15	-.12	1	0	4	-.014	4	-.004	3
429		6	max	716.836	2	4.022	6	5.544	4	0	1	0	12	-.001	15
430			min	-848.479	3	.934	15	-.12	1	0	4	-.012	4	-.005	4
431		7	max	716.665	2	3.254	6	6.083	4	0	1	0	12	-.002	15
432			min	-848.606	3	.754	15	-.12	1	0	4	-.01	4	-.007	4
433		8	max	716.495	2	2.487	6	6.621	4	0	1	0	12	-.002	15
434			min	-848.734	3	.573	15	-.12	1	0	4	-.007	4	-.008	4
435		9	max	716.325	2	1.72	6	7.16	4	0	1	0	12	-.002	15
436			min	-848.862	3	.393	15	-.12	1	0	4	-.004	4	-.009	4
437		10	max	716.154	2	.965	2	7.699	4	0	1	0	12	-.002	15
438			min	-848.99	3	.213	15	-.12	1	0	4	-.001	4	-.009	4
439		11	max	715.984	2	.367	2	8.238	4	0	1	.002	5	-.002	15
440			min	-849.117	3	-.15	3	-.12	1	0	4	0	1	-.01	4
441		12	max	715.814	2	-.148	15	8.776	4	0	1	.006	5	-.002	15
442			min	-849.245	3	-.598	3	-.12	1	0	4	0	1	-.01	4
443		13	max	715.643	2	-.328	15	9.315	4	0	1	.01	5	-.002	15
444			min	-849.373	3	-1.35	4	-.12	1	0	4	0	1	-.009	4
445		14	max	715.473	2	-.509	15	9.854	4	0	1	.014	4	-.002	15
446			min	-849.501	3	-2.117	4	-.12	1	0	4	0	1	-.008	4
447		15	max	715.303	2	-.689	15	10.393	4	0	1	.018	4	-.002	15
448			min	-849.628	3	-2.884	4	-.12	1	0	4	0	1	-.007	4
449		16	max	715.132	2	-.869	15	10.931	4	0	1	.022	4	-.001	15
450			min	-849.756	3	-3.652	4	-.12	1	0	4	-.001	1	-.006	4
451		17	max	714.962	2	-1.05	15	11.47	4	0	1	.027	4	-.001	15
452			min	-849.884	3	-4.419	4	-.12	1	0	4	-.001	1	-.004	4
453		18	max	714.792	2	-1.23	15	12.009	4	0	1	.032	4	0	15
454			min	-850.012	3	-5.186	4	-.12	1	0	4	-.001	1	-.002	4
455		19	max	714.621	2	-1.411	15	12.548	4	0	1	.037	4	0	1
456			min	-850.14	3	-5.953	4	-.12	1	0	4	-.001	1	0	1
457	M12	1	max	1049.665	1	0	1	7.349	1	0	1	.027	4	0	1
458			min	-179.306	3	0	1	-230.144	4	0	1	0	1	0	1
459		2	max	1049.835	1	0	1	7.349	1	0	1	0	5	0	1
460			min	-179.178	3	0	1	-230.292	4	0	1	0	3	0	1
461		3	max	1050.005	1	0	1	7.349	1	0	1	0	1	0	1
462			min	-179.051	3	0	1	-230.439	4	0	1	-.026	4	0	1
463		4	max	1050.176	1	0	1	7.349	1	0	1	.002	1	0	1
464			min	-178.923	3	0	1	-230.587	4	0	1	-.053	4	0	1
465		5	max	1050.346	1	0	1	7.349	1	0	1	.003	1	0	1
466			min	-178.795	3	0	1	-230.735	4	0	1	-.079	4	0	1
467		6	max	1050.516	1	0	1	7.349	1	0	1	.003	1	0	1
468			min	-178.667	3	0	1	-230.882	4	0	1	-.106	4	0	1
469		7	max	1050.687	1	0	1	7.349	1	0	1	.004	1	0	1
470			min	-178.54	3	0	1	-231.03	4	0	1	-.132	4	0	1
471		8	max	1050.857	1	0	1	7.349	1	0	1	.005	1	0	1
472			min	-178.412	3	0	1	-231.178	4	0	1	-.159	4	0	1
473		9	max	1051.027	1	0	1	7.349	1	0	1	.006	1	0	1
474			min	-178.284	3	0	1	-231.325	4	0	1	-.185	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1051.198	1	0	1	7.349	1	0	1	.007	1	0	1
476			min	-178.156	3	0	1	-231.473	4	0	1	-.212	4	0	1
477		11	max	1051.368	1	0	1	7.349	1	0	1	.008	1	0	1
478			min	-178.029	3	0	1	-231.621	4	0	1	-.238	4	0	1
479		12	max	1051.538	1	0	1	7.349	1	0	1	.008	1	0	1
480			min	-177.901	3	0	1	-231.768	4	0	1	-.265	4	0	1
481		13	max	1051.709	1	0	1	7.349	1	0	1	.009	1	0	1
482			min	-177.773	3	0	1	-231.916	4	0	1	-.292	4	0	1
483		14	max	1051.879	1	0	1	7.349	1	0	1	.01	1	0	1
484			min	-177.645	3	0	1	-232.063	4	0	1	-.318	4	0	1
485		15	max	1052.049	1	0	1	7.349	1	0	1	.011	1	0	1
486			min	-177.517	3	0	1	-232.211	4	0	1	-.345	4	0	1
487		16	max	1052.22	1	0	1	7.349	1	0	1	.012	1	0	1
488			min	-177.39	3	0	1	-232.359	4	0	1	-.371	4	0	1
489		17	max	1052.39	1	0	1	7.349	1	0	1	.013	1	0	1
490			min	-177.262	3	0	1	-232.506	4	0	1	-.398	4	0	1
491		18	max	1052.56	1	0	1	7.349	1	0	1	.014	1	0	1
492			min	-177.134	3	0	1	-232.654	4	0	1	-.425	4	0	1
493		19	max	1052.731	1	0	1	7.349	1	0	1	.014	1	0	1
494			min	-177.006	3	0	1	-232.802	4	0	1	-.452	4	0	1
495	M1	1	max	144.019	1	853.436	3	40.849	5	0	2	.169	1	0	15
496			min	-11.836	5	-470.935	2	-70.801	1	0	3	-.08	5	-.015	2
497		2	max	144.624	1	852.463	3	42.09	5	0	2	.131	1	.233	2
498			min	-11.553	5	-472.233	2	-70.801	1	0	3	-.058	5	-.451	3
499		3	max	523.744	3	577.612	2	10.038	5	0	3	.094	1	.47	2
500			min	-305.353	2	-628.545	3	-70.47	1	0	2	-.036	5	-.883	3
501		4	max	524.198	3	576.314	2	11.279	5	0	3	.057	1	.166	2
502			min	-304.747	2	-629.519	3	-70.47	1	0	2	-.03	5	-.551	3
503		5	max	524.652	3	575.016	2	12.521	5	0	3	.02	1	-.003	15
504			min	-304.142	2	-630.492	3	-70.47	1	0	2	-.024	5	-.219	3
505		6	max	525.106	3	573.717	2	13.762	5	0	3	-.001	12	.114	3
506			min	-303.537	2	-631.466	3	-70.47	1	0	2	-.021	4	-.441	2
507		7	max	525.56	3	572.419	2	15.004	5	0	3	-.004	12	.448	3
508			min	-302.931	2	-632.44	3	-70.47	1	0	2	-.055	1	-.743	2
509		8	max	526.014	3	571.121	2	16.245	5	0	3	0	15	.782	3
510			min	-302.326	2	-633.413	3	-70.47	1	0	2	-.092	1	-1.045	2
511		9	max	537.955	3	51.964	2	48.879	5	0	9	.056	1	.913	3
512			min	-242.464	2	.392	15	-107.629	1	0	3	-.107	5	-1.195	2
513		10	max	538.409	3	50.666	2	50.12	5	0	9	0	10	.889	3
514			min	-241.859	2	-.001	5	-107.629	1	0	3	-.081	4	-1.222	2
515		11	max	538.863	3	49.368	2	51.362	5	0	9	-.004	12	.867	3
516			min	-241.254	2	-1.629	4	-107.629	1	0	3	-.067	4	-1.249	2
517		12	max	550.63	3	409.845	3	129.105	5	0	2	.091	1	.757	3
518			min	-181.317	2	-679.51	2	-69.048	1	0	3	-.181	5	-1.107	2
519		13	max	551.084	3	408.871	3	130.347	5	0	2	.055	1	.541	3
520			min	-180.711	2	-680.808	2	-69.048	1	0	3	-.113	5	-.748	2
521		14	max	551.538	3	407.898	3	131.588	5	0	2	.018	1	.325	3
522			min	-180.106	2	-682.106	2	-69.048	1	0	3	-.044	5	-.389	2
523		15	max	551.992	3	406.924	3	132.83	5	0	2	.026	5	.11	3
524			min	-179.501	2	-683.405	2	-69.048	1	0	3	-.018	1	-.046	1
525		16	max	552.446	3	405.95	3	134.071	5	0	2	.096	5	.332	2
526			min	-178.895	2	-684.703	2	-69.048	1	0	3	-.055	1	-.104	3
527		17	max	552.9	3	404.977	3	135.313	5	0	2	.167	5	.694	2
528			min	-178.29	2	-686.001	2	-69.048	1	0	3	-.091	1	-.318	3
529		18	max	21.982	5	683.417	2	-4.935	12	0	5	.166	5	.35	2
530			min	-144.944	1	-322.542	3	-98.15	4	0	2	-.13	1	-.157	3
531		19	max	22.265	5	682.119	2	-4.935	12	0	5	.124	5	.013	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-144.339	1	-323.516	3	-96.908	4	0	2	-.17	1	-.011	2
533	M5	max	317.823	1	2836.605	3	78.575	5	0	1	0	1	.031	2
534		min	10.332	12	-1616.019	2	0	1	0	4	-.169	4	0	15
535		max	318.428	1	2835.631	3	79.816	5	0	1	0	1	.884	2
536		min	10.635	12	-1617.317	2	0	1	0	4	-.127	4	-1.494	3
537		max	1656.964	3	1673.241	2	49.814	4	0	4	0	1	1.698	2
538		min	-1017.359	2	-1946.903	3	0	1	0	1	-.085	4	-2.932	3
539		max	1657.418	3	1671.943	2	51.055	4	0	4	0	1	.816	2
540		min	-1016.754	2	-1947.877	3	0	1	0	1	-.058	4	-1.905	3
541		max	1657.872	3	1670.644	2	52.297	4	0	4	0	1	.017	9
542		min	-1016.148	2	-1948.851	3	0	1	0	1	-.031	4	-.877	3
543		max	1658.326	3	1669.346	2	53.538	4	0	4	0	1	.152	3
544		min	-1015.543	2	-1949.824	3	0	1	0	1	-.003	5	-.948	2
545		max	1658.78	3	1668.048	2	54.78	4	0	4	.025	4	1.181	3
546		min	-1014.938	2	-1950.798	3	0	1	0	1	0	1	-1.828	2
547		max	1659.234	3	1666.75	2	56.021	4	0	4	.055	4	2.211	3
548		min	-1014.332	2	-1951.772	3	0	1	0	1	0	1	-2.708	2
549		max	1674.081	3	174.887	2	159.549	4	0	1	0	1	2.546	3
550		min	-886.126	2	.39	15	0	1	0	1	-.157	4	-3.088	2
551		max	1674.535	3	173.589	2	160.79	4	0	1	0	1	2.46	3
552		min	-885.521	2	-.001	15	0	1	0	1	-.073	4	-3.18	2
553		max	1674.99	3	172.291	2	162.032	4	0	1	.012	4	2.375	3
554		min	-884.915	2	-1.533	6	0	1	0	1	0	1	-3.271	2
555		max	1690.185	3	1240.126	3	184.212	4	0	1	0	1	2.083	3
556		min	-756.859	2	-2019.914	2	0	1	0	4	-.261	4	-2.928	2
557		max	1690.639	3	1239.152	3	185.454	4	0	1	0	1	1.429	3
558		min	-756.254	2	-2021.212	2	0	1	0	4	-.163	4	-1.862	2
559		max	1691.093	3	1238.179	3	186.695	4	0	1	0	1	.775	3
560		min	-755.648	2	-2022.51	2	0	1	0	4	-.065	4	-.795	2
561		max	1691.547	3	1237.205	3	187.936	4	0	1	.034	4	.273	2
562		min	-755.043	2	-2023.808	2	0	1	0	4	0	1	-.002	13
563		max	1692.001	3	1236.231	3	189.178	4	0	1	.133	4	1.341	2
564		min	-754.438	2	-2025.107	2	0	1	0	4	0	1	-.531	3
565		max	1692.455	3	1235.258	3	190.419	4	0	1	.233	4	2.41	2
566		min	-753.832	2	-2026.405	2	0	1	0	4	0	1	-1.183	3
567		max	-11.341	12	2312.576	2	0	1	0	4	.263	4	1.241	2
568		min	-317.788	1	-1121.843	3	-21.815	5	0	1	0	1	-.619	3
569		max	-11.039	12	2311.278	2	0	1	0	4	.252	4	.021	2
570		min	-317.183	1	-1122.817	3	-20.573	5	0	1	0	1	-.027	3
571	M9	max	144.019	1	853.436	3	70.801	1	0	3	-.012	12	0	15
572		min	7.29	12	-470.935	2	5.125	12	0	4	-.169	1	-.015	2
573		max	144.624	1	852.463	3	70.801	1	0	3	-.01	12	.233	2
574		min	7.593	12	-472.233	2	5.125	12	0	4	-.131	1	-.451	3
575		max	523.744	3	577.612	2	70.47	1	0	2	-.007	12	.47	2
576		min	-305.353	2	-628.545	3	5.093	12	0	3	-.094	1	-.883	3
577		max	524.198	3	576.314	2	70.47	1	0	2	-.004	12	.166	2
578		min	-304.747	2	-629.519	3	5.093	12	0	3	-.057	1	-.551	3
579		max	524.652	3	575.016	2	70.47	1	0	2	-.002	12	-.004	15
580		min	-304.142	2	-630.492	3	5.093	12	0	3	-.031	4	-.219	3
581		max	525.106	3	573.717	2	70.47	1	0	2	.018	1	.114	3
582		min	-303.537	2	-631.466	3	5.093	12	0	3	-.014	5	-.441	2
583		max	525.56	3	572.419	2	70.47	1	0	2	.055	1	.448	3
584		min	-302.931	2	-632.44	3	5.093	12	0	3	-.002	5	-.743	2
585		max	526.014	3	571.121	2	70.47	1	0	2	.092	1	.782	3
586		min	-302.326	2	-633.413	3	5.093	12	0	3	.007	12	-1.045	2
587		max	537.955	3	51.964	2	107.629	1	0	3	-.004	12	.913	3
588		min	-242.464	2	.399	15	7.37	12	0	9	-.128	4	-1.195	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	538.409	3	50.666	2	107.629	1	0	3	0	1	.889	3
590		min	-241.859	2	.007	15	7.37	12	0	9	-.081	4	-1.222	2
591	11	max	538.863	3	49.368	2	107.629	1	0	3	.057	1	.867	3
592		min	-241.254	2	-1.579	6	7.37	12	0	9	-.047	5	-1.249	2
593	12	max	550.63	3	409.845	3	154.865	4	0	3	-.006	12	.757	3
594		min	-181.317	2	-679.51	2	4.477	12	0	2	-.215	4	-1.107	2
595	13	max	551.084	3	408.871	3	156.107	4	0	3	-.004	12	.541	3
596		min	-180.711	2	-680.808	2	4.477	12	0	2	-.133	4	-.748	2
597	14	max	551.538	3	407.898	3	157.348	4	0	3	-.001	12	.325	3
598		min	-180.106	2	-682.106	2	4.477	12	0	2	-.051	4	-.389	2
599	15	max	551.992	3	406.924	3	158.589	4	0	3	.033	4	.11	3
600		min	-179.501	2	-683.405	2	4.477	12	0	2	.001	12	-.046	1
601	16	max	552.446	3	405.95	3	159.831	4	0	3	.117	4	.332	2
602		min	-178.895	2	-684.703	2	4.477	12	0	2	.003	12	-.104	3
603	17	max	552.9	3	404.977	3	161.072	4	0	3	.201	4	.694	2
604		min	-178.29	2	-686.001	2	4.477	12	0	2	.006	12	-.318	3
605	18	max	-7.239	12	683.417	2	76.087	1	0	2	.214	4	.35	2
606		min	-144.944	1	-322.542	3	-70.88	5	0	3	.008	12	-.157	3
607	19	max	-6.937	12	682.119	2	76.087	1	0	2	.186	4	.013	3
608		min	-144.339	1	-323.516	3	-69.639	5	0	3	.011	12	-.011	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.128	2	.009	3	1.052e-2	2	NC	1	NC	1
2			min	-493	4	-.025	3	-.005	2	-2.259e-3	3	NC	1	NC	1
3		2	max	0	1	.204	3	.019	1	1.177e-2	2	NC	4	NC	1
4			min	-493	4	-.001	9	-.011	5	-2.156e-3	3	862.968	3	NC	1
5		3	max	0	1	.39	3	.046	1	1.302e-2	2	NC	5	NC	2
6			min	-493	4	-.067	1	-.014	5	-2.053e-3	3	476.288	3	4322.017	1
7		4	max	0	1	.505	3	.068	1	1.427e-2	2	NC	5	NC	3
8			min	-493	4	-.107	1	-.01	5	-1.95e-3	3	373.726	3	2903.015	1
9		5	max	0	1	.533	3	.079	1	1.551e-2	2	NC	5	NC	3
10			min	-493	4	-.104	1	-.003	5	-1.847e-3	3	354.775	3	2503.965	1
11		6	max	0	1	.477	3	.075	1	1.676e-2	2	NC	5	NC	3
12			min	-493	4	-.061	1	0	10	-1.745e-3	3	394.07	3	2632.904	1
13		7	max	0	1	.355	3	.057	1	1.801e-2	2	NC	4	NC	2
14			min	-493	4	-.004	9	-.003	10	-1.642e-3	3	520.902	3	3443.315	1
15		8	max	0	1	.199	3	.031	1	1.926e-2	2	NC	1	NC	2
16			min	-493	4	.003	15	-.007	10	-1.539e-3	3	884.814	3	6381.868	1
17		9	max	0	1	.224	2	.029	3	2.051e-2	2	NC	4	NC	1
18			min	-493	4	.004	15	-.014	2	-1.436e-3	3	2049.642	2	NC	1
19		10	max	0	1	.263	2	.028	3	2.175e-2	2	NC	3	NC	1
20			min	-493	4	-.008	3	-.02	2	-1.333e-3	3	1460.15	2	NC	1
21		11	max	0	12	.224	2	.029	3	2.051e-2	2	NC	4	NC	1
22			min	-493	4	.004	15	-.014	2	-1.436e-3	3	2049.642	2	NC	1
23		12	max	0	12	.199	3	.031	1	1.926e-2	2	NC	1	NC	2
24			min	-493	4	.002	15	-.009	5	-1.539e-3	3	884.814	3	6381.868	1
25		13	max	0	12	.355	3	.057	1	1.801e-2	2	NC	4	NC	2
26			min	-493	4	-.004	9	-.003	5	-1.642e-3	3	520.902	3	3443.315	1
27		14	max	0	12	.477	3	.075	1	1.676e-2	2	NC	5	NC	3
28			min	-493	4	-.061	1	0	10	-1.745e-3	3	394.07	3	2632.904	1
29		15	max	0	12	.533	3	.079	1	1.551e-2	2	NC	5	NC	3
30			min	-493	4	-.104	1	.002	10	-1.847e-3	3	354.775	3	2503.965	1
31		16	max	0	12	.505	3	.068	1	1.427e-2	2	NC	5	NC	3
32			min	-493	4	-.107	1	.002	10	-1.95e-3	3	373.726	3	2903.015	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.39	3	.046	1	1.302e-2	2	NC	5	NC	2
34		min	-.493	4	-.067	1	0	10	-2.053e-3	3	476.288	3	4322.017	1
35	18	max	0	12	.204	3	.02	4	1.177e-2	2	NC	4	NC	1
36		min	-.493	4	-.001	9	-.002	10	-2.156e-3	3	862.968	3	9889.161	4
37	19	max	0	12	.128	2	.009	3	1.052e-2	2	NC	1	NC	1
38		min	-.493	4	-.025	3	-.005	2	-2.259e-3	3	NC	1	NC	1
39	M14	1	max	0	.274	3	.008	3	6.049e-3	2	NC	1	NC	1
40		min	-.379	4	-.401	2	-.005	2	-4.854e-3	3	NC	1	NC	1
41	2	max	0	1	.522	3	.013	1	7.123e-3	2	NC	5	NC	1
42		min	-.379	4	-.63	2	-.017	5	-5.807e-3	3	799.083	3	NC	1
43	3	max	0	1	.735	3	.036	1	8.197e-3	2	NC	5	NC	2
44		min	-.379	4	-.831	2	-.021	5	-6.759e-3	3	429.406	3	5552.319	1
45	4	max	0	1	.891	3	.057	1	9.271e-3	2	NC	5	NC	3
46		min	-.379	4	-.987	2	-.015	5	-7.711e-3	3	321.337	3	3466.749	1
47	5	max	0	1	.975	3	.069	1	1.035e-2	2	NC	15	NC	3
48		min	-.379	4	-1.088	2	-.003	5	-8.664e-3	3	282.498	3	2871.76	1
49	6	max	0	1	.989	3	.067	1	1.142e-2	2	NC	15	NC	3
50		min	-.379	4	-1.131	2	0	10	-9.616e-3	3	271.431	2	2941.989	1
51	7	max	0	1	.943	3	.053	1	1.249e-2	2	NC	15	NC	2
52		min	-.38	4	-1.124	2	-.003	10	-1.057e-2	3	273.966	2	3774.936	1
53	8	max	0	1	.861	3	.034	4	1.357e-2	2	NC	5	NC	2
54		min	-.38	4	-1.083	2	-.006	10	-1.152e-2	3	290.29	2	6073.58	4
55	9	max	0	1	.776	3	.025	3	1.464e-2	2	NC	5	NC	1
56		min	-.38	4	-1.034	2	-.013	2	-1.247e-2	3	313.177	2	9115.8	4
57	10	max	0	1	.736	3	.025	3	1.572e-2	2	NC	5	NC	1
58		min	-.38	4	-1.008	2	-.018	2	-1.343e-2	3	326.43	2	NC	1
59	11	max	0	12	.776	3	.025	3	1.464e-2	2	NC	5	NC	1
60		min	-.38	4	-1.034	2	-.017	5	-1.247e-2	3	313.177	2	NC	1
61	12	max	0	12	.861	3	.029	1	1.357e-2	2	NC	5	NC	2
62		min	-.38	4	-1.083	2	-.02	5	-1.152e-2	3	290.29	2	6873.721	1
63	13	max	0	12	.943	3	.053	1	1.249e-2	2	NC	15	NC	2
64		min	-.38	4	-1.124	2	-.013	5	-1.057e-2	3	273.966	2	3774.936	1
65	14	max	0	12	.989	3	.067	1	1.142e-2	2	NC	15	NC	3
66		min	-.38	4	-1.131	2	0	5	-9.616e-3	3	271.431	2	2941.989	1
67	15	max	0	12	.975	3	.069	1	1.035e-2	2	NC	15	NC	3
68		min	-.38	4	-1.088	2	.002	10	-8.664e-3	3	282.498	3	2871.76	1
69	16	max	0	12	.891	3	.057	1	9.271e-3	2	NC	5	NC	3
70		min	-.38	4	-.987	2	.001	10	-7.711e-3	3	321.337	3	3466.749	1
71	17	max	0	12	.735	3	.036	1	8.197e-3	2	NC	5	NC	2
72		min	-.38	4	-.831	2	0	10	-6.759e-3	3	429.406	3	5552.319	1
73	18	max	0	12	.522	3	.023	4	7.123e-3	2	NC	5	NC	1
74		min	-.38	4	-.63	2	-.002	10	-5.807e-3	3	799.083	3	8405.624	4
75	19	max	0	12	.274	3	.008	3	6.049e-3	2	NC	1	NC	1
76		min	-.38	4	-.401	2	-.005	2	-4.854e-3	3	NC	1	NC	1
77	M15	1	max	0	.28	3	.008	3	4.13e-3	3	NC	1	NC	1
78		min	-.316	4	-.4	2	-.005	2	-6.294e-3	2	NC	1	NC	1
79	2	max	0	12	.447	3	.013	1	4.941e-3	3	NC	5	NC	1
80		min	-.316	4	-.684	2	-.024	5	-7.416e-3	2	697.969	2	7752.289	5
81	3	max	0	12	.594	3	.036	1	5.752e-3	3	NC	5	NC	2
82		min	-.316	4	-.93	2	-.03	5	-8.539e-3	2	373.555	2	5530.708	1
83	4	max	0	12	.71	3	.057	1	6.562e-3	3	NC	5	NC	3
84		min	-.316	4	-1.114	2	-.022	5	-9.662e-3	2	277.624	2	3454.64	1
85	5	max	0	12	.786	3	.069	1	7.373e-3	3	NC	15	NC	3
86		min	-.316	4	-1.22	2	-.006	5	-1.078e-2	2	241.546	2	2861.218	1
87	6	max	0	12	.823	3	.068	1	8.184e-3	3	NC	15	NC	3
88		min	-.316	4	-1.249	2	0	10	-1.191e-2	2	233.308	2	2928.934	1
89	7	max	0	12	.823	3	.053	1	8.994e-3	3	NC	15	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.316	4	-1.212	2	-.002	10	-1.303e-2	2	243.979	2	3750.822	1
91	8	max	0	12	.799	3	.041	4	9.805e-3	3	NC	5	NC	2
92		min	-.316	4	-1.133	2	-.006	10	-1.415e-2	2	270.363	2	4982.019	4
93	9	max	0	12	.768	3	.029	4	1.062e-2	3	NC	5	NC	1
94		min	-.316	4	-1.048	2	-.012	2	-1.527e-2	2	305.776	2	7170.396	4
95	10	max	0	1	.751	3	.024	3	1.143e-2	3	NC	5	NC	1
96		min	-.316	4	-1.006	2	-.017	2	-1.64e-2	2	326.723	2	NC	1
97	11	max	0	1	.768	3	.024	3	1.062e-2	3	NC	5	NC	1
98		min	-.316	4	-1.048	2	-.023	5	-1.527e-2	2	305.776	2	8460.222	5
99	12	max	0	1	.799	3	.029	1	9.805e-3	3	NC	5	NC	2
100		min	-.316	4	-1.133	2	-.027	5	-1.415e-2	2	270.363	2	6787.747	1
101	13	max	0	1	.823	3	.053	1	8.994e-3	3	NC	15	NC	2
102		min	-.316	4	-1.212	2	-.018	5	-1.303e-2	2	243.979	2	3750.822	1
103	14	max	0	1	.823	3	.068	1	8.184e-3	3	NC	15	NC	3
104		min	-.316	4	-1.249	2	-.002	5	-1.191e-2	2	233.308	2	2928.934	1
105	15	max	0	1	.786	3	.069	1	7.373e-3	3	NC	15	NC	3
106		min	-.316	4	-1.22	2	.002	10	-1.078e-2	2	241.546	2	2861.218	1
107	16	max	0	1	.71	3	.057	1	6.562e-3	3	NC	5	NC	3
108		min	-.316	4	-1.114	2	.002	10	-9.662e-3	2	277.624	2	3454.64	1
109	17	max	0	1	.594	3	.044	4	5.752e-3	3	NC	5	NC	2
110		min	-.316	4	-.93	2	0	10	-8.539e-3	2	373.555	2	4469.721	4
111	18	max	0	1	.447	3	.03	4	4.941e-3	3	NC	5	NC	1
112		min	-.316	4	-.684	2	-.002	10	-7.416e-3	2	697.969	2	6526.051	4
113	19	max	0	1	.28	3	.008	3	4.13e-3	3	NC	1	NC	1
114		min	-.315	4	-.4	2	-.005	2	-6.294e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.114	.007	3	7.589e-3	3	NC	1	NC	1
116		min	-.127	4	-.095	3	-.004	2	-8.853e-3	2	NC	1	NC	1
117	2	max	0	12	.003	4	.019	1	8.678e-3	3	NC	4	NC	1
118		min	-.127	4	-.051	2	-.018	5	-9.717e-3	2	1197.503	2	NC	1
119	3	max	0	12	.024	3	.046	1	9.767e-3	3	NC	5	NC	2
120		min	-.127	4	-.182	2	-.024	5	-1.058e-2	2	668.24	2	4321.081	1
121	4	max	0	12	.048	3	.068	1	1.086e-2	3	NC	5	NC	3
122		min	-.127	4	-.256	2	-.018	5	-1.145e-2	2	535.264	2	2893.884	1
123	5	max	0	12	.04	3	.08	1	1.195e-2	3	NC	5	NC	3
124		min	-.127	4	-.262	2	-.008	5	-1.231e-2	2	527.035	2	2488.23	1
125	6	max	0	12	.004	12	.076	1	1.303e-2	3	NC	5	NC	3
126		min	-.127	4	-.202	2	.002	10	-1.317e-2	2	627.363	2	2604.368	1
127	7	max	0	12	.003	4	.059	1	1.412e-2	3	NC	4	NC	2
128		min	-.127	4	-.09	2	0	10	-1.404e-2	2	970.019	2	3376	1
129	8	max	0	12	.063	1	.032	1	1.521e-2	3	NC	4	NC	2
130		min	-.127	4	-.131	3	-.005	10	-1.49e-2	2	2905.648	2	6101.187	1
131	9	max	0	12	.167	2	.021	3	1.63e-2	3	NC	4	NC	1
132		min	-.127	4	-.193	3	-.01	2	-1.577e-2	2	2009.921	3	NC	1
133	10	max	0	1	.221	2	.021	3	1.739e-2	3	NC	4	NC	1
134		min	-.127	4	-.221	3	-.016	2	-1.663e-2	2	1571.119	3	NC	1
135	11	max	0	1	.167	2	.021	3	1.63e-2	3	NC	4	NC	1
136		min	-.127	4	-.193	3	-.014	5	-1.577e-2	2	2009.921	3	NC	1
137	12	max	0	1	.063	1	.032	1	1.521e-2	3	NC	4	NC	2
138		min	-.127	4	-.131	3	-.015	5	-1.49e-2	2	2905.648	2	6101.187	1
139	13	max	0	1	.003	6	.059	1	1.412e-2	3	NC	4	NC	2
140		min	-.126	4	-.09	2	-.007	5	-1.404e-2	2	970.019	2	3376	1
141	14	max	0	1	.004	12	.076	1	1.303e-2	3	NC	5	NC	3
142		min	-.126	4	-.202	2	.002	10	-1.317e-2	2	627.363	2	2604.368	1
143	15	max	0	1	.04	3	.08	1	1.195e-2	3	NC	5	NC	3
144		min	-.126	4	-.262	2	.003	10	-1.231e-2	2	527.035	2	2488.23	1
145	16	max	0	1	.048	3	.068	1	1.086e-2	3	NC	5	NC	3
146		min	-.126	4	-.256	2	.003	10	-1.145e-2	2	535.264	2	2893.884	1



Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.024	3	.046	1	9.767e-3	3	NC	5	NC	2
148			min	-.126	4	-.182	2	.001	10	-1.058e-2	2	668.24	2	4321.081	1
149		18	max	0	1	.003	6	.026	4	8.678e-3	3	NC	4	NC	1
150			min	-.126	4	-.051	2	-.001	10	-9.717e-3	2	1197.503	2	7415.043	4
151		19	max	0	1	.114	2	.007	3	7.589e-3	3	NC	1	NC	1
152			min	-.126	4	-.095	3	-.004	2	-8.853e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.008	2	.006	1	1.329e-3	5	NC	1	NC	1
154			min	-.009	3	-.013	3	-.465	4	-1.452e-4	1	7807.748	2	134.819	4
155		2	max	.006	2	.007	2	.005	1	1.393e-3	5	NC	1	NC	1
156			min	-.009	3	-.012	3	-.427	4	-1.363e-4	1	8894.8	2	146.753	4
157		3	max	.006	2	.006	2	.005	1	1.457e-3	5	NC	1	NC	1
158			min	-.008	3	-.012	3	-.39	4	-1.274e-4	1	NC	1	160.918	4
159		4	max	.006	2	.005	2	.004	1	1.521e-3	5	NC	1	NC	1
160			min	-.008	3	-.011	3	-.353	4	-1.185e-4	1	NC	1	177.892	4
161		5	max	.005	2	.004	2	.004	1	1.585e-3	5	NC	1	NC	1
162			min	-.007	3	-.011	3	-.316	4	-1.097e-4	1	NC	1	198.461	4
163		6	max	.005	2	.003	2	.003	1	1.649e-3	5	NC	1	NC	1
164			min	-.007	3	-.01	3	-.28	4	-1.008e-4	1	NC	1	223.716	4
165		7	max	.005	2	.003	2	.003	1	1.713e-3	5	NC	1	NC	1
166			min	-.006	3	-.01	3	-.246	4	-9.192e-5	1	NC	1	255.198	4
167		8	max	.004	2	.002	2	.002	1	1.777e-3	5	NC	1	NC	1
168			min	-.006	3	-.009	3	-.213	4	-8.305e-5	1	NC	1	295.154	4
169		9	max	.004	2	.001	2	.002	1	1.841e-3	5	NC	1	NC	1
170			min	-.005	3	-.008	3	-.181	4	-7.418e-5	1	NC	1	346.959	4
171		10	max	.003	2	0	2	.002	1	1.908e-3	4	NC	1	NC	1
172			min	-.005	3	-.008	3	-.151	4	-6.531e-5	1	NC	1	415.875	4
173		11	max	.003	2	0	2	.001	1	1.974e-3	4	NC	1	NC	1
174			min	-.004	3	-.007	3	-.123	4	-5.644e-5	1	NC	1	510.491	4
175		12	max	.003	2	0	2	.001	1	2.041e-3	4	NC	1	NC	1
176			min	-.004	3	-.006	3	-.097	4	-4.756e-5	1	NC	1	645.611	4
177		13	max	.002	2	0	15	0	1	2.108e-3	4	NC	1	NC	1
178			min	-.003	3	-.006	3	-.074	4	-3.869e-5	1	NC	1	848.606	4
179		14	max	.002	2	0	15	0	1	2.175e-3	4	NC	1	NC	1
180			min	-.003	3	-.005	3	-.053	4	-2.982e-5	1	NC	1	1174.869	4
181		15	max	.002	2	0	15	0	1	2.242e-3	4	NC	1	NC	1
182			min	-.002	3	-.004	3	-.036	4	-2.095e-5	1	NC	1	1751.209	4
183		16	max	.001	2	0	15	0	1	2.309e-3	4	NC	1	NC	1
184			min	-.002	3	-.003	3	-.021	4	-1.208e-5	1	NC	1	2924.704	4
185		17	max	0	2	0	15	0	1	2.376e-3	4	NC	1	NC	1
186			min	-.001	3	-.002	3	-.011	4	-3.207e-6	1	NC	1	5962.187	4
187		18	max	0	2	0	15	0	1	2.443e-3	4	NC	1	NC	1
188			min	0	3	-.001	3	-.003	4	-1.096e-7	3	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.51e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	6.241e-7	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-2.406e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.127e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.012	4	1.086e-5	1	NC	1	NC	1
194			min	0	2	-.002	6	0	12	-6.602e-5	5	NC	1	7611.506	4
195		3	max	0	3	0	15	.023	4	4.861e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	1.706e-6	12	NC	1	3950.737	4
197		4	max	.001	3	-.001	15	.033	4	1.035e-3	4	NC	1	NC	1
198			min	-.001	2	-.005	6	0	12	2.679e-6	12	NC	1	2732.348	4
199		5	max	.002	3	-.002	15	.043	4	1.585e-3	4	NC	1	NC	1
200			min	-.001	2	-.007	6	0	12	3.652e-6	12	NC	1	2123.727	4
201		6	max	.002	3	-.002	15	.051	4	2.134e-3	4	NC	1	NC	1
202			min	-.002	2	-.009	6	0	12	4.626e-6	12	NC	1	1758.183	4
203		7	max	.002	3	-.002	15	.06	4	2.684e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.01	6	0	12	5.599e-6	12	8761.035	6	1513.338	4
205		8	max	.003	3	-.003	15	.068	4	3.233e-3	4	NC	1	NC	1
206			min	-.002	2	-.012	6	0	12	6.572e-6	12	7855.521	6	1336.618	4
207		9	max	.003	3	-.003	15	.075	4	3.782e-3	4	NC	2	NC	1
208			min	-.003	2	-.012	6	0	12	7.545e-6	12	7318.958	6	1201.634	4
209		10	max	.004	3	-.003	15	.083	4	4.332e-3	4	NC	2	NC	1
210			min	-.003	2	-.013	6	0	12	8.518e-6	12	7057.573	6	1093.655	4
211		11	max	.004	3	-.003	15	.09	4	4.881e-3	4	NC	2	NC	1
212			min	-.003	2	-.013	6	0	12	9.492e-6	12	7032.758	6	1003.806	4
213		12	max	.005	3	-.003	15	.098	4	5.431e-3	4	NC	2	NC	1
214			min	-.004	2	-.013	6	0	12	1.046e-5	12	7246.245	6	926.442	4
215		13	max	.005	3	-.003	15	.106	4	5.98e-3	4	NC	1	NC	1
216			min	-.004	2	-.012	6	0	12	1.144e-5	12	7742.667	6	857.842	4
217		14	max	.005	3	-.002	15	.114	4	6.529e-3	4	NC	1	NC	1
218			min	-.005	2	-.011	6	0	12	1.241e-5	12	8632.801	6	795.512	4
219		15	max	.006	3	-.002	15	.123	4	7.079e-3	4	NC	1	NC	1
220			min	-.005	2	-.009	6	0	12	1.338e-5	12	NC	1	737.784	4
221		16	max	.006	3	-.001	15	.132	4	7.628e-3	4	NC	1	NC	1
222			min	-.005	2	-.007	6	0	12	1.436e-5	12	NC	1	683.569	4
223		17	max	.007	3	0	15	.143	4	8.177e-3	4	NC	1	NC	1
224			min	-.006	2	-.005	1	0	12	1.533e-5	12	NC	1	632.202	4
225		18	max	.007	3	0	15	.155	4	8.727e-3	4	NC	1	NC	1
226			min	-.006	2	-.004	1	0	12	1.63e-5	12	NC	1	583.314	4
227		19	max	.007	3	0	5	.169	4	9.276e-3	4	NC	1	NC	1
228			min	-.006	2	-.002	1	0	12	1.728e-5	12	NC	1	536.748	4
229	M4	1	max	.003	1	.006	2	0	12	1.89e-4	4	NC	1	NC	2
230			min	0	3	-.008	3	-.169	4	3.28e-6	12	NC	1	147.086	4
231		2	max	.002	1	.005	2	0	12	1.89e-4	4	NC	1	NC	2
232			min	0	3	-.007	3	-.155	4	3.28e-6	12	NC	1	159.865	4
233		3	max	.002	1	.005	2	0	12	1.89e-4	4	NC	1	NC	2
234			min	0	3	-.007	3	-.142	4	3.28e-6	12	NC	1	175.076	4
235		4	max	.002	1	.005	2	0	12	1.89e-4	4	NC	1	NC	2
236			min	0	3	-.006	3	-.128	4	3.28e-6	12	NC	1	193.353	4
237		5	max	.002	1	.005	2	0	12	1.89e-4	4	NC	1	NC	2
238			min	0	3	-.006	3	-.115	4	3.28e-6	12	NC	1	215.551	4
239		6	max	.002	1	.004	2	0	12	1.89e-4	4	NC	1	NC	2
240			min	0	3	-.005	3	-.102	4	3.28e-6	12	NC	1	242.862	4
241		7	max	.002	1	.004	2	0	12	1.89e-4	4	NC	1	NC	2
242			min	0	3	-.005	3	-.09	4	3.28e-6	12	NC	1	276.974	4
243		8	max	.002	1	.004	2	0	12	1.89e-4	4	NC	1	NC	1
244			min	0	3	-.005	3	-.077	4	3.28e-6	12	NC	1	320.349	4
245		9	max	.001	1	.003	2	0	12	1.89e-4	4	NC	1	NC	1
246			min	0	3	-.004	3	-.066	4	3.28e-6	12	NC	1	376.695	4
247		10	max	.001	1	.003	2	0	12	1.89e-4	4	NC	1	NC	1
248			min	0	3	-.004	3	-.055	4	3.28e-6	12	NC	1	451.806	4
249		11	max	.001	1	.003	2	0	12	1.89e-4	4	NC	1	NC	1
250			min	0	3	-.003	3	-.045	4	3.28e-6	12	NC	1	555.162	4
251		12	max	0	1	.002	2	0	12	1.89e-4	4	NC	1	NC	1
252			min	0	3	-.003	3	-.035	4	3.28e-6	12	NC	1	703.157	4
253		13	max	0	1	.002	2	0	12	1.89e-4	4	NC	1	NC	1
254			min	0	3	-.003	3	-.027	4	3.28e-6	12	NC	1	926.216	4
255		14	max	0	1	.002	2	0	12	1.89e-4	4	NC	1	NC	1
256			min	0	3	-.002	3	-.019	4	3.28e-6	12	NC	1	1286.225	4
257		15	max	0	1	.001	2	0	12	1.89e-4	4	NC	1	NC	1
258			min	0	3	-.002	3	-.013	4	3.28e-6	12	NC	1	1925.815	4
259		16	max	0	1	0	2	0	12	1.89e-4	4	NC	1	NC	1
260			min	0	3	-.001	3	-.008	4	3.28e-6	12	NC	1	3239.163	4



Company : Schletter, Inc.  
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Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	1.89e-4	4	NC	1	NC	1
262			min	0	3	0	3	-.004	4	3.28e-6	12	NC	1	6687.233	4
263		18	max	0	1	0	2	0	12	1.89e-4	4	NC	1	NC	1
264			min	0	3	0	3	-.001	4	3.28e-6	12	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.89e-4	4	NC	1	NC	1
266			min	0	1	0	1	0	1	3.28e-6	12	NC	1	NC	1
267	M6	1	max	.021	2	.028	2	0	1	1.39e-3	4	NC	4	NC	1
268			min	-.03	3	-.04	3	-.469	4	0	1	1548.684	3	133.646	4
269		2	max	.02	2	.026	2	0	1	1.452e-3	4	NC	4	NC	1
270			min	-.029	3	-.038	3	-.431	4	0	1	1643.693	3	145.478	4
271		3	max	.019	2	.024	2	0	1	1.514e-3	4	NC	4	NC	1
272			min	-.027	3	-.036	3	-.393	4	0	1	1751.077	3	159.522	4
273		4	max	.018	2	.021	2	0	1	1.577e-3	4	NC	4	NC	1
274			min	-.025	3	-.033	3	-.356	4	0	1	1873.366	3	176.352	4
275		5	max	.017	2	.019	2	0	1	1.639e-3	4	NC	4	NC	1
276			min	-.024	3	-.031	3	-.319	4	0	1	2013.809	3	196.748	4
277		6	max	.015	2	.017	2	0	1	1.701e-3	4	NC	4	NC	1
278			min	-.022	3	-.029	3	-.283	4	0	1	2176.662	3	221.79	4
279		7	max	.014	2	.015	2	0	1	1.764e-3	4	NC	4	NC	1
280			min	-.02	3	-.026	3	-.248	4	0	1	2367.601	3	253.008	4
281		8	max	.013	2	.013	2	0	1	1.826e-3	4	NC	1	NC	1
282			min	-.019	3	-.024	3	-.214	4	0	1	2594.372	3	292.63	4
283		9	max	.012	2	.011	2	0	1	1.888e-3	4	NC	1	NC	1
284			min	-.017	3	-.022	3	-.182	4	0	1	2867.824	3	344.004	4
285		10	max	.011	2	.009	2	0	1	1.951e-3	4	NC	1	NC	1
286			min	-.015	3	-.02	3	-.152	4	0	1	3203.636	3	412.351	4
287		11	max	.009	2	.007	2	0	1	2.013e-3	4	NC	1	NC	1
288			min	-.013	3	-.017	3	-.124	4	0	1	3625.334	3	506.191	4
289		12	max	.008	2	.006	2	0	1	2.075e-3	4	NC	1	NC	1
290			min	-.012	3	-.015	3	-.098	4	0	1	4169.903	3	640.212	4
291		13	max	.007	2	.004	2	0	1	2.138e-3	4	NC	1	NC	1
292			min	-.01	3	-.013	3	-.075	4	0	1	4898.998	3	841.576	4
293		14	max	.006	2	.003	2	0	1	2.2e-3	4	NC	1	NC	1
294			min	-.008	3	-.011	3	-.054	4	0	1	5923.621	3	1165.256	4
295		15	max	.005	2	.002	2	0	1	2.262e-3	4	NC	1	NC	1
296			min	-.007	3	-.008	3	-.036	4	0	1	7465.81	3	1737.129	4
297		16	max	.004	2	.001	2	0	1	2.325e-3	4	NC	1	NC	1
298			min	-.005	3	-.006	3	-.022	4	0	1	NC	1	2901.816	4
299		17	max	.002	2	0	2	0	1	2.387e-3	4	NC	1	NC	1
300			min	-.003	3	-.004	3	-.011	4	0	1	NC	1	5917.75	4
301		18	max	.001	2	0	2	0	1	2.449e-3	4	NC	1	NC	1
302			min	-.002	3	-.002	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.512e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-6.126e-4	4	NC	1	NC	1
307		2	max	.001	3	0	2	.012	4	0	1	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	-7.605e-5	5	NC	1	NC	1
309		3	max	.003	3	0	2	.023	4	4.607e-4	4	NC	1	NC	1
310			min	-.002	2	-.006	3	0	1	0	1	NC	1	7736.856	14
311		4	max	.004	3	-.001	2	.033	4	9.973e-4	4	NC	1	NC	1
312			min	-.004	2	-.008	3	0	1	0	1	NC	1	5354.367	14
313		5	max	.005	3	-.002	15	.043	4	1.534e-3	4	NC	1	NC	1
314			min	-.005	2	-.01	3	0	1	0	1	9946.526	3	4164.994	14
315		6	max	.006	3	-.002	15	.051	4	2.07e-3	4	NC	1	NC	1
316			min	-.006	2	-.012	3	0	1	0	1	8452.306	3	3451.328	14
317		7	max	.008	3	-.003	15	.06	4	2.607e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.007	2	-.014	3	0	1	0	1	7559.546	3	2973.938	14
319	8	max	-.009	3	-.003	15	.067	4	3.144e-3	4	NC	1	NC	1
320		min	-.008	2	-.015	3	0	1	0	1	7032.912	3	2629.95	14
321	9	max	.01	3	-.003	15	.075	4	3.68e-3	4	NC	1	NC	1
322		min	-.01	2	-.016	3	0	1	0	1	6763.375	3	2367.713	14
323	10	max	.011	3	-.003	15	.082	4	4.217e-3	4	NC	1	NC	1
324		min	-.011	2	-.016	3	0	1	0	1	6702.259	3	2158.368	14
325	11	max	.013	3	-.003	15	.089	4	4.753e-3	4	NC	1	NC	1
326		min	-.012	2	-.016	3	0	1	0	1	6837.741	3	1984.505	14
327	12	max	.014	3	-.003	15	.097	4	5.29e-3	4	NC	1	NC	1
328		min	-.013	2	-.016	3	0	1	0	1	7189.427	3	1835.019	14
329	13	max	.015	3	-.003	15	.104	4	5.827e-3	4	NC	1	NC	1
330		min	-.015	2	-.015	3	0	1	0	1	7749.893	4	1702.563	14
331	14	max	.017	3	-.003	15	.112	4	6.363e-3	4	NC	1	NC	1
332		min	-.016	2	-.014	3	0	1	0	1	8640.572	4	1582.186	14
333	15	max	.018	3	-.002	15	.12	4	6.9e-3	4	NC	1	NC	1
334		min	-.017	2	-.012	3	0	1	0	1	NC	1	1470.552	14
335	16	max	.019	3	-.002	15	.13	4	7.437e-3	4	NC	1	NC	1
336		min	-.018	2	-.011	3	0	1	0	1	NC	1	1365.474	14
337	17	max	.02	3	-.001	15	.14	4	7.973e-3	4	NC	1	NC	1
338		min	-.019	2	-.009	3	0	1	0	1	NC	1	1265.598	14
339	18	max	.022	3	0	15	.151	4	8.51e-3	4	NC	1	NC	1
340		min	-.021	2	-.007	3	0	1	0	1	NC	1	1170.182	14
341	19	max	.023	3	0	15	.164	4	9.046e-3	4	NC	1	NC	1
342		min	-.022	2	-.005	3	0	1	0	1	NC	1	1078.915	14
343	M8	1	max	.007	2	.021	2	0	1.15e-4	4	NC	1	NC	1
344		min	-.002	3	-.023	3	-.164	4	0	1	NC	1	151.214	4
345	2	max	.007	2	.019	2	0	1	1.15e-4	4	NC	1	NC	1
346		min	-.001	3	-.022	3	-.151	4	0	1	NC	1	164.357	4
347	3	max	.006	2	.018	2	0	1	1.15e-4	4	NC	1	NC	1
348		min	-.001	3	-.021	3	-.138	4	0	1	NC	1	180.003	4
349	4	max	.006	2	.017	2	0	1	1.15e-4	4	NC	1	NC	1
350		min	-.001	3	-.019	3	-.125	4	0	1	NC	1	198.8	4
351	5	max	.006	2	.016	2	0	1	1.15e-4	4	NC	1	NC	1
352		min	-.001	3	-.018	3	-.112	4	0	1	NC	1	221.63	4
353	6	max	.005	2	.015	2	0	1	1.15e-4	4	NC	1	NC	1
354		min	-.001	3	-.017	3	-.099	4	0	1	NC	1	249.719	4
355	7	max	.005	2	.014	2	0	1	1.15e-4	4	NC	1	NC	1
356		min	-.001	3	-.016	3	-.087	4	0	1	NC	1	284.801	4
357	8	max	.004	2	.013	2	0	1	1.15e-4	4	NC	1	NC	1
358		min	0	3	-.014	3	-.075	4	0	1	NC	1	329.411	4
359	9	max	.004	2	.011	2	0	1	1.15e-4	4	NC	1	NC	1
360		min	0	3	-.013	3	-.064	4	0	1	NC	1	387.36	4
361	10	max	.004	2	.01	2	0	1	1.15e-4	4	NC	1	NC	1
362		min	0	3	-.012	3	-.053	4	0	1	NC	1	464.609	4
363	11	max	.003	2	.009	2	0	1	1.15e-4	4	NC	1	NC	1
364		min	0	3	-.01	3	-.043	4	0	1	NC	1	570.907	4
365	12	max	.003	2	.008	2	0	1	1.15e-4	4	NC	1	NC	1
366		min	0	3	-.009	3	-.034	4	0	1	NC	1	723.115	4
367	13	max	.002	2	.007	2	0	1	1.15e-4	4	NC	1	NC	1
368		min	0	3	-.008	3	-.026	4	0	1	NC	1	952.525	4
369	14	max	.002	2	.006	2	0	1	1.15e-4	4	NC	1	NC	1
370		min	0	3	-.006	3	-.019	4	0	1	NC	1	1322.787	4
371	15	max	.002	2	.005	2	0	1	1.15e-4	4	NC	1	NC	1
372		min	0	3	-.005	3	-.013	4	0	1	NC	1	1980.599	4
373	16	max	.001	2	.003	2	0	1	1.15e-4	4	NC	1	NC	1
374		min	0	3	-.004	3	-.007	4	0	1	NC	1	3331.383	4





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.002	2	0	1	1.15e-4	4	NC	1	NC	1
376			min	0	3	-.003	3	-.004	4	0	1	NC	1	6877.798	4
377		18	max	0	2	.001	2	0	1	1.15e-4	4	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	1.15e-4	4	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.008	2	0	12	1.389e-3	4	NC	1	NC	1
382			min	-.009	3	-.013	3	-.468	4	1.139e-5	12	7807.748	2	133.904	4
383		2	max	.006	2	.007	2	0	12	1.451e-3	4	NC	1	NC	1
384			min	-.009	3	-.012	3	-.43	4	1.072e-5	12	8894.8	2	145.76	4
385		3	max	.006	2	.006	2	0	12	1.513e-3	4	NC	1	NC	1
386			min	-.008	3	-.012	3	-.392	4	1.006e-5	12	NC	1	159.831	4
387		4	max	.006	2	.005	2	0	12	1.575e-3	4	NC	1	NC	1
388			min	-.008	3	-.011	3	-.355	4	9.389e-6	12	NC	1	176.693	4
389		5	max	.005	2	.004	2	0	12	1.636e-3	4	NC	1	NC	1
390			min	-.007	3	-.011	3	-.318	4	8.721e-6	12	NC	1	197.129	4
391		6	max	.005	2	.003	2	0	12	1.698e-3	4	NC	1	NC	1
392			min	-.007	3	-.01	3	-.282	4	8.054e-6	12	NC	1	222.22	4
393		7	max	.005	2	.003	2	0	12	1.76e-3	4	NC	1	NC	1
394			min	-.006	3	-.01	3	-.247	4	7.386e-6	12	NC	1	253.499	4
395		8	max	.004	2	.002	2	0	12	1.822e-3	4	NC	1	NC	1
396			min	-.006	3	-.009	3	-.214	4	6.719e-6	12	NC	1	293.2	4
397		9	max	.004	2	.001	2	0	12	1.883e-3	4	NC	1	NC	1
398			min	-.005	3	-.008	3	-.182	4	6.051e-6	12	NC	1	344.676	4
399		10	max	.003	2	0	2	0	12	1.945e-3	4	NC	1	NC	1
400			min	-.005	3	-.008	3	-.152	4	5.384e-6	12	NC	1	413.159	4
401		11	max	.003	2	0	2	0	12	2.007e-3	4	NC	1	NC	1
402			min	-.004	3	-.007	3	-.124	4	4.716e-6	12	NC	1	507.187	4
403		12	max	.003	2	0	2	0	12	2.069e-3	4	NC	1	NC	1
404			min	-.004	3	-.006	3	-.098	4	4.049e-6	12	NC	1	641.48	4
405		13	max	.002	2	0	2	0	12	2.131e-3	4	NC	1	NC	1
406			min	-.003	3	-.006	3	-.074	4	3.325e-6	10	NC	1	843.256	4
407		14	max	.002	2	0	2	0	12	2.192e-3	4	NC	1	NC	1
408			min	-.003	3	-.005	3	-.054	4	2.515e-6	10	NC	1	1167.61	4
409		15	max	.002	2	0	15	0	12	2.254e-3	4	NC	1	NC	1
410			min	-.002	3	-.004	3	-.036	4	1.705e-6	10	NC	1	1740.699	4
411		16	max	.001	2	0	15	0	12	2.316e-3	4	NC	1	NC	1
412			min	-.002	3	-.003	3	-.022	4	8.948e-7	10	NC	1	2907.947	4
413		17	max	0	2	0	15	0	12	2.378e-3	4	NC	1	NC	1
414			min	-.001	3	-.002	3	-.011	4	8.466e-8	10	NC	1	5930.868	4
415		18	max	0	2	0	15	0	12	2.439e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.003	4	-5.664e-6	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.501e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.454e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	4.966e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.098e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.012	4	-7.326e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-6.954e-5	4	NC	1	7639.237	4
423		3	max	0	3	0	15	.023	4	4.707e-4	4	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-2.67e-5	1	NC	1	3967.05	4
425		4	max	.001	3	-.001	15	.033	4	1.011e-3	4	NC	1	NC	1
426			min	-.001	2	-.006	4	0	1	-4.253e-5	1	NC	1	2745.145	4
427		5	max	.002	3	-.002	15	.042	4	1.551e-3	4	NC	1	NC	1
428			min	-.001	2	-.008	4	0	1	-5.836e-5	1	NC	1	2135.003	4
429		6	max	.002	3	-.002	15	.051	4	2.092e-3	4	NC	1	NC	1
430			min	-.002	2	-.01	4	0	1	-7.419e-5	1	9857.228	4	1768.749	4
431		7	max	.002	3	-.003	15	.059	4	2.632e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.011	4	0	1	-9.002e-5	1	8468.211	4	1523.604	4
433		8	max	.003	3	-.003	15	.067	4	3.172e-3	4	NC	1	NC	1
434			min	-.002	2	-.012	4	0	1	-1.059e-4	1	7611.547	4	1346.818	4
435		9	max	.003	3	-.003	15	.075	4	3.712e-3	4	NC	2	NC	1
436			min	-.003	2	-.013	4	-.001	1	-1.217e-4	1	7106.078	4	1211.904	4
437		10	max	.004	3	-.003	15	.082	4	4.253e-3	4	NC	2	NC	1
438			min	-.003	2	-.014	4	-.001	1	-1.375e-4	1	6863.981	4	1104.07	4
439		11	max	.004	3	-.003	15	.089	4	4.793e-3	4	NC	2	NC	1
440			min	-.003	2	-.014	4	-.002	1	-1.533e-4	1	6849.667	4	1014.396	4
441		12	max	.005	3	-.003	15	.097	4	5.333e-3	4	NC	2	NC	1
442			min	-.004	2	-.014	4	-.002	1	-1.692e-4	1	7066.137	4	937.2	4
443		13	max	.005	3	-.003	15	.104	4	5.873e-3	4	NC	1	NC	1
444			min	-.004	2	-.013	4	-.002	1	-1.85e-4	1	7557.892	4	868.73	4
445		14	max	.005	3	-.003	15	.112	4	6.414e-3	4	NC	1	NC	1
446			min	-.005	2	-.012	4	-.003	1	-2.008e-4	1	8433.904	4	806.466	4
447		15	max	.006	3	-.003	15	.121	4	6.954e-3	4	NC	1	NC	1
448			min	-.005	2	-.01	4	-.003	1	-2.167e-4	1	9935.318	4	748.719	4
449		16	max	.006	3	-.002	15	.13	4	7.494e-3	4	NC	1	NC	1
450			min	-.005	2	-.008	4	-.004	1	-2.325e-4	1	NC	1	694.388	4
451		17	max	.007	3	-.002	15	.141	4	8.035e-3	4	NC	1	NC	1
452			min	-.006	2	-.006	4	-.004	1	-2.483e-4	1	NC	1	642.798	4
453		18	max	.007	3	-.001	15	.152	4	8.575e-3	4	NC	1	NC	1
454			min	-.006	2	-.004	1	-.005	1	-2.642e-4	1	NC	1	593.583	4
455		19	max	.007	3	0	10	.166	4	9.115e-3	4	NC	1	NC	1
456			min	-.006	2	-.002	1	-.005	1	-2.8e-4	1	NC	1	546.594	4
457	M12	1	max	.003	1	.006	2	.005	1	1.678e-4	5	NC	1	NC	2
458			min	0	3	-.008	3	-.166	4	-4.572e-5	1	NC	1	149.784	4
459		2	max	.002	1	.005	2	.005	1	1.678e-4	5	NC	1	NC	2
460			min	0	3	-.007	3	-.152	4	-4.572e-5	1	NC	1	162.797	4
461		3	max	.002	1	.005	2	.004	1	1.678e-4	5	NC	1	NC	2
462			min	0	3	-.007	3	-.139	4	-4.572e-5	1	NC	1	178.287	4
463		4	max	.002	1	.005	2	.004	1	1.678e-4	5	NC	1	NC	2
464			min	0	3	-.006	3	-.126	4	-4.572e-5	1	NC	1	196.898	4
465		5	max	.002	1	.005	2	.004	1	1.678e-4	5	NC	1	NC	2
466			min	0	3	-.006	3	-.113	4	-4.572e-5	1	NC	1	219.503	4
467		6	max	.002	1	.004	2	.003	1	1.678e-4	5	NC	1	NC	2
468			min	0	3	-.005	3	-.1	4	-4.572e-5	1	NC	1	247.314	4
469		7	max	.002	1	.004	2	.003	1	1.678e-4	5	NC	1	NC	2
470			min	0	3	-.005	3	-.088	4	-4.572e-5	1	NC	1	282.051	4
471		8	max	.002	1	.004	2	.002	1	1.678e-4	5	NC	1	NC	1
472			min	0	3	-.005	3	-.076	4	-4.572e-5	1	NC	1	326.22	4
473		9	max	.001	1	.003	2	.002	1	1.678e-4	5	NC	1	NC	1
474			min	0	3	-.004	3	-.065	4	-4.572e-5	1	NC	1	383.598	4
475		10	max	.001	1	.003	2	.002	1	1.678e-4	5	NC	1	NC	1
476			min	0	3	-.004	3	-.054	4	-4.572e-5	1	NC	1	460.085	4
477		11	max	.001	1	.003	2	.001	1	1.678e-4	5	NC	1	NC	1
478			min	0	3	-.003	3	-.044	4	-4.572e-5	1	NC	1	565.333	4
479		12	max	0	1	.002	2	.001	1	1.678e-4	5	NC	1	NC	1
480			min	0	3	-.003	3	-.035	4	-4.572e-5	1	NC	1	716.037	4
481		13	max	0	1	.002	2	0	1	1.678e-4	5	NC	1	NC	1
482			min	0	3	-.003	3	-.026	4	-4.572e-5	1	NC	1	943.18	4
483		14	max	0	1	.002	2	0	1	1.678e-4	5	NC	1	NC	1
484			min	0	3	-.002	3	-.019	4	-4.572e-5	1	NC	1	1309.779	4
485		15	max	0	1	.001	2	0	1	1.678e-4	5	NC	1	NC	1
486			min	0	3	-.002	3	-.013	4	-4.572e-5	1	NC	1	1961.077	4
487		16	max	0	1	0	2	0	1	1.678e-4	5	NC	1	NC	1
488			min	0	3	-.001	3	-.008	4	-4.572e-5	1	NC	1	3298.466	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	1.678e-4	5	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-4.572e-5	1	NC	1	6809.643	4
491		18	max	0	1	0	2	0	1	1.678e-4	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-4.572e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	1.678e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-4.572e-5	1	NC	1	NC	1
495	M1	1	max	.009	3	.128	2	.493	4	1.001e-2	2	NC	1	NC	1
496			min	-.005	2	-.025	3	0	12	-2.165e-2	3	NC	1	NC	1
497		2	max	.009	3	.061	2	.479	4	6.147e-3	4	NC	4	NC	1
498			min	-.005	2	-.01	3	-.004	1	-1.071e-2	3	1723.773	2	NC	1
499		3	max	.009	3	.014	3	.465	4	1.065e-2	4	NC	5	NC	1
500			min	-.005	2	-.011	2	-.006	1	-1.182e-4	3	832.384	2	8362.807	5
501		4	max	.009	3	.054	3	.45	4	9.207e-3	4	NC	5	NC	1
502			min	-.005	2	-.091	2	-.005	1	-4.383e-3	3	526.922	2	5989.934	5
503		5	max	.009	3	.106	3	.435	4	7.885e-3	2	NC	5	NC	1
504			min	-.005	2	-.174	2	-.004	1	-8.648e-3	3	381.192	2	4792.133	5
505		6	max	.009	3	.162	3	.419	4	1.181e-2	2	NC	15	NC	1
506			min	-.005	2	-.255	2	-.002	1	-1.291e-2	3	300.777	2	4065.819	5
507		7	max	.008	3	.216	3	.403	4	1.574e-2	2	NC	15	NC	1
508			min	-.005	2	-.327	2	0	3	-1.718e-2	3	253.241	2	3555.667	4
509		8	max	.008	3	.261	3	.386	4	1.967e-2	2	NC	15	NC	1
510			min	-.005	2	-.384	2	0	12	-2.144e-2	3	225.093	2	3176.637	4
511		9	max	.008	3	.29	3	.369	4	2.222e-2	2	9571.308	15	NC	1
512			min	-.005	2	-.42	2	0	1	-2.17e-2	3	210.428	2	2927.323	4
513		10	max	.008	3	.3	3	.35	4	2.385e-2	2	9370.222	15	NC	1
514			min	-.005	2	-.432	2	0	12	-1.93e-2	3	206.125	2	2843.565	4
515		11	max	.008	3	.293	3	.329	4	2.548e-2	2	9570.869	15	NC	1
516			min	-.005	2	-.42	2	0	12	-1.689e-2	3	211.131	2	2886.725	4
517		12	max	.008	3	.268	3	.306	4	2.452e-2	2	NC	15	NC	1
518			min	-.005	2	-.382	2	0	1	-1.431e-2	3	227.214	2	3063.472	4
519		13	max	.007	3	.229	3	.28	4	1.966e-2	2	NC	15	NC	1
520			min	-.004	2	-.323	2	0	1	-1.145e-2	3	258.364	2	3570.717	4
521		14	max	.007	3	.178	3	.253	4	1.48e-2	2	NC	15	NC	1
522			min	-.004	2	-.248	2	0	12	-8.6e-3	3	311.65	2	4662.041	4
523		15	max	.007	3	.121	3	.224	4	9.942e-3	2	NC	5	NC	1
524			min	-.004	2	-.166	2	0	12	-5.745e-3	3	403.421	2	7072.951	4
525		16	max	.007	3	.062	3	.196	4	7.732e-3	4	NC	5	NC	1
526			min	-.004	2	-.083	2	0	12	-2.891e-3	3	573.436	2	NC	1
527		17	max	.007	3	.005	3	.169	4	8.845e-3	4	NC	5	NC	1
528			min	-.004	2	-.006	2	0	12	-3.628e-5	3	936.988	2	NC	1
529		18	max	.007	3	.057	2	.146	4	7.799e-3	2	NC	4	NC	1
530			min	-.004	2	-.047	3	0	12	-3.16e-3	3	1988.677	2	NC	1
531		19	max	.007	3	.114	2	.126	4	1.567e-2	2	NC	1	NC	1
532			min	-.004	2	-.095	3	0	1	-6.424e-3	3	NC	1	NC	1
533	M5	1	max	.028	3	.263	2	.493	4	0	1	NC	1	NC	1
534			min	-.02	2	-.008	3	0	1	-5.707e-6	4	NC	1	NC	1
535		2	max	.028	3	.124	2	.482	4	5.463e-3	4	NC	5	NC	1
536			min	-.02	2	.002	15	0	1	0	1	831.281	2	NC	1
537		3	max	.028	3	.043	3	.469	4	1.076e-2	4	NC	5	NC	1
538			min	-.02	2	-.033	2	0	1	0	1	390.39	2	6877.909	4
539		4	max	.028	3	.139	3	.454	4	8.767e-3	4	NC	15	NC	1
540			min	-.019	2	-.222	2	0	1	0	1	238.37	2	5266.991	4
541		5	max	.027	3	.273	3	.437	4	6.774e-3	4	8664.31	15	NC	1
542			min	-.019	2	-.427	2	0	1	0	1	167.453	2	4477.826	4
543		6	max	.027	3	.424	3	.42	4	4.78e-3	4	6660.406	15	NC	1
544			min	-.019	2	-.63	2	0	1	0	1	129.252	2	3985.255	4
545		7	max	.026	3	.572	3	.403	4	2.786e-3	4	5505.215	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.018	2	-.815	2	0	1	0	1	107.115	2	3598.125	4
547		8	max	.026	3	.697	3	.386	4	7.92e-4	4	4834.428	15	NC	1
548			min	-.018	2	-.963	2	0	1	0	1	94.212	2	3228.929	4
549		9	max	.025	3	.777	3	.369	4	0	1	4490.777	15	NC	1
550			min	-.018	2	-1.057	2	0	1	-3.936e-6	5	87.585	2	2921.508	4
551		10	max	.024	3	.806	3	.35	4	0	1	4387.303	15	NC	1
552			min	-.018	2	-1.089	2	0	1	-3.799e-6	5	85.646	2	2864.124	4
553		11	max	.024	3	.786	3	.329	4	0	1	4490.979	15	NC	1
554			min	-.017	2	-1.057	2	0	1	-3.663e-6	5	87.896	2	2921.417	4
555		12	max	.023	3	.718	3	.307	4	6.303e-4	4	4834.897	15	NC	1
556			min	-.017	2	-.959	2	0	1	0	1	95.241	2	3009.686	4
557		13	max	.023	3	.608	3	.281	4	2.218e-3	4	5506.142	15	NC	1
558			min	-.017	2	-.803	2	0	1	0	1	109.799	2	3503.415	4
559		14	max	.022	3	.47	3	.252	4	3.805e-3	4	6662.176	15	NC	1
560			min	-.016	2	-.609	2	0	1	0	1	135.335	2	4811.445	4
561		15	max	.022	3	.316	3	.222	4	5.392e-3	4	8667.751	15	NC	1
562			min	-.016	2	-.4	2	0	1	0	1	180.79	2	8477.086	4
563		16	max	.021	3	.16	3	.192	4	6.98e-3	4	NC	15	NC	1
564			min	-.016	2	-.196	2	0	1	0	1	268.73	2	NC	1
565		17	max	.021	3	.014	3	.165	4	8.567e-3	4	NC	5	NC	1
566			min	-.016	2	-.019	2	0	1	0	1	465.922	2	NC	1
567		18	max	.021	3	.114	2	.143	4	4.35e-3	4	NC	5	NC	1
568			min	-.016	2	-.109	3	0	1	0	1	1037.446	2	NC	1
569		19	max	.021	3	.221	2	.127	4	0	1	NC	1	NC	1
570			min	-.016	2	-.221	3	0	1	-3.163e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.128	2	.493	4	2.165e-2	3	NC	1	NC	1
572			min	-.005	2	-.025	3	0	1	-1.001e-2	2	NC	1	NC	1
573		2	max	.009	3	.061	2	.482	4	1.071e-2	3	NC	4	NC	1
574			min	-.005	2	-.01	3	0	12	-4.916e-3	2	1723.773	2	NC	1
575		3	max	.009	3	.014	3	.468	4	1.073e-2	4	NC	5	NC	1
576			min	-.005	2	-.011	2	0	12	-3.29e-5	10	832.384	2	7194.667	4
577		4	max	.009	3	.054	3	.453	4	8.488e-3	5	NC	5	NC	1
578			min	-.005	2	-.091	2	0	12	-3.955e-3	2	526.922	2	5401.047	4
579		5	max	.009	3	.106	3	.437	4	8.648e-3	3	NC	5	NC	1
580			min	-.005	2	-.174	2	0	12	-7.885e-3	2	381.192	2	4509.327	4
581		6	max	.009	3	.162	3	.42	4	1.291e-2	3	NC	15	NC	1
582			min	-.005	2	-.255	2	0	12	-1.181e-2	2	300.777	2	3959.286	4
583		7	max	.008	3	.216	3	.403	4	1.718e-2	3	NC	15	NC	1
584			min	-.005	2	-.327	2	0	1	-1.574e-2	2	253.241	2	3553.417	4
585		8	max	.008	3	.261	3	.386	4	2.144e-2	3	NC	15	NC	1
586			min	-.005	2	-.384	2	0	1	-1.967e-2	2	225.093	2	3202.214	4
587		9	max	.008	3	.29	3	.369	4	2.17e-2	3	9550.408	15	NC	1
588			min	-.005	2	-.42	2	0	12	-2.222e-2	2	210.428	2	2919.722	4
589		10	max	.008	3	.3	3	.35	4	1.93e-2	3	9349.797	15	NC	1
590			min	-.005	2	-.432	2	0	1	-2.385e-2	2	206.125	2	2844.622	4
591		11	max	.008	3	.293	3	.329	4	1.689e-2	3	9549.93	15	NC	1
592			min	-.005	2	-.42	2	0	1	-2.548e-2	2	211.131	2	2895.799	4
593		12	max	.008	3	.268	3	.307	4	1.431e-2	3	NC	15	NC	1
594			min	-.005	2	-.382	2	0	12	-2.452e-2	2	227.214	2	3040.026	4
595		13	max	.007	3	.229	3	.281	4	1.145e-2	3	NC	15	NC	1
596			min	-.004	2	-.323	2	0	10	-1.966e-2	2	258.364	2	3568.936	4
597		14	max	.007	3	.178	3	.252	4	8.6e-3	3	NC	15	NC	1
598			min	-.004	2	-.248	2	-.001	1	-1.48e-2	2	311.65	2	4789.497	5
599		15	max	.007	3	.121	3	.222	4	5.745e-3	3	NC	5	NC	1
600			min	-.004	2	-.166	2	-.003	1	-9.942e-3	2	403.421	2	7696.1	5
601		16	max	.007	3	.062	3	.193	4	6.904e-3	5	NC	5	NC	1
602			min	-.004	2	-.083	2	-.005	1	-5.083e-3	2	573.436	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.007	3	.005	3	.166	4	8.652e-3	4	NC	5	NC	1
604		min	-.004	2	-.006	2	-.005	1	-3.831e-4	1	936.988	2	NC	1
605	18	max	.007	3	.057	2	.144	4	4.213e-3	5	NC	4	NC	1
606		min	-.004	2	-.047	3	-.004	1	-7.799e-3	2	1988.677	2	NC	1
607	19	max	.007	3	.114	2	.127	4	6.424e-3	3	NC	1	NC	1
608		min	-.004	2	-.095	3	0	12	-1.567e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

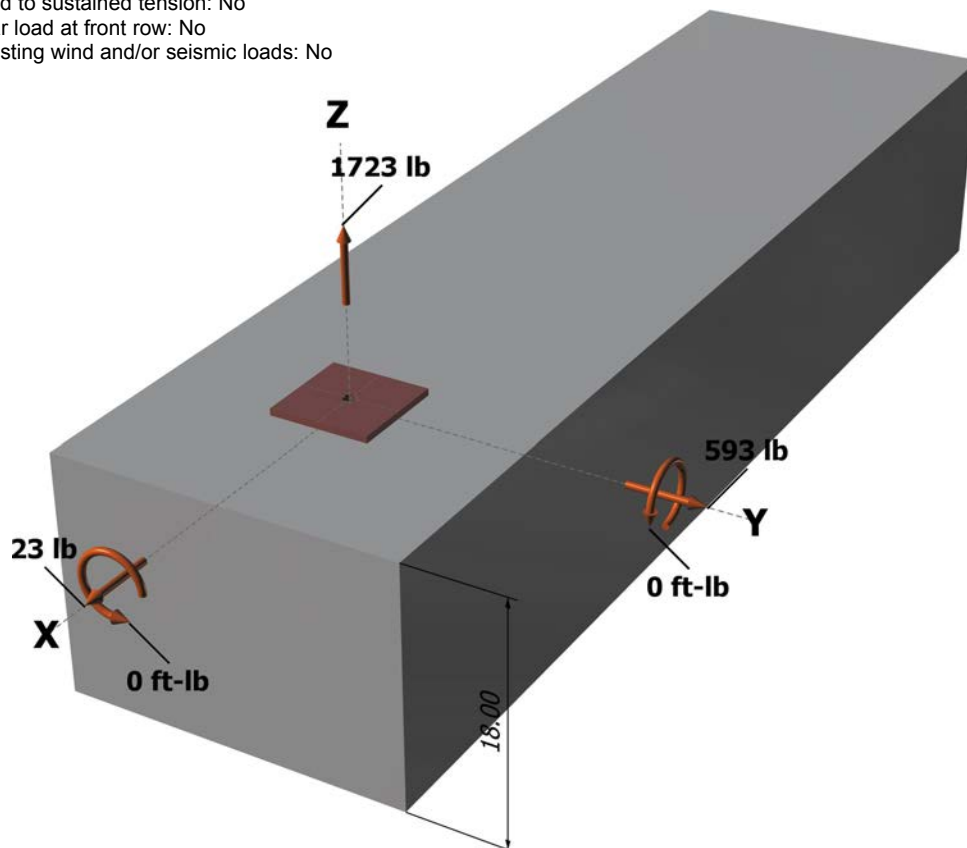
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1723  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 37-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

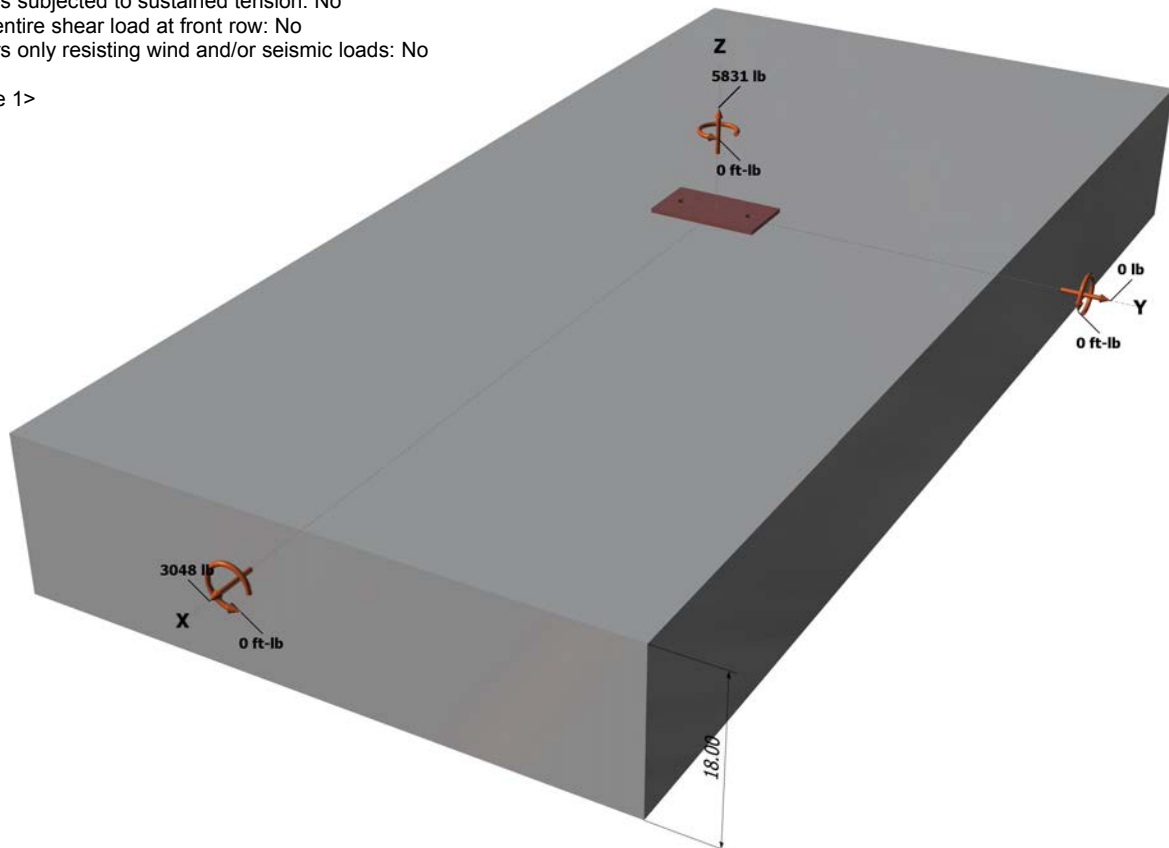
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

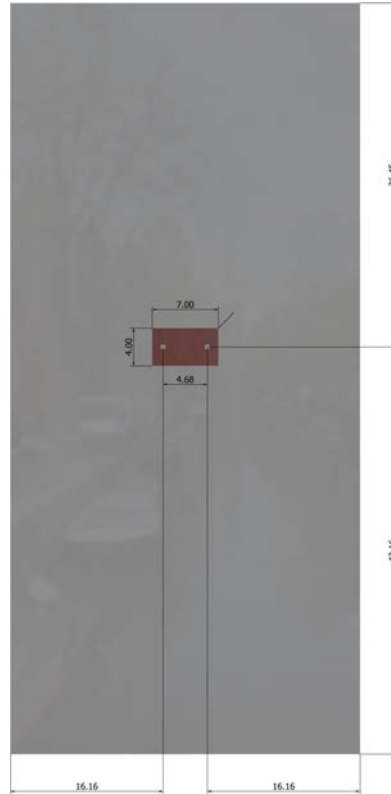
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2915.5	1524.0	0.0	1524.0
2	2915.5	1524.0	0.0	1524.0
Sum	5831.0	3048.0	0.0	3048.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5831  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
666.00	648.00	1.000	0.969	1.000	1.000	15593	0.70	10875

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	16.16	24369

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
872.64	1175.16	1.000	1.000	1.000	24369	0.70	25334

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2916	6071	0.48	Pass	
Concrete breakout	5831	10231	0.57	Pass	
<b>Adhesive</b>	<b>5831</b>	<b>8093</b>	<b>0.72</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1524</b>	<b>3156</b>	<b>0.48</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3048	10875	0.28	Pass	
Concrete breakout y-	1524	25334	0.06	Pass	
Pryout	3048	20601	0.15	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.72	0.48	120.3 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.