

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.041 k-ft
P_n =	0.227 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	12%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.340 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

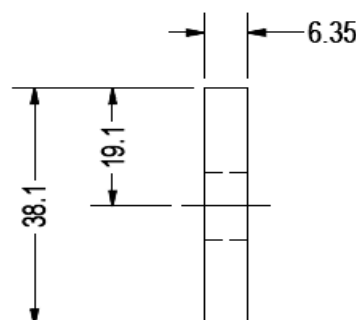
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.997 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.004 k-ft
P_n =	0.207 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	11%



A cross brace kit is required every 18 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

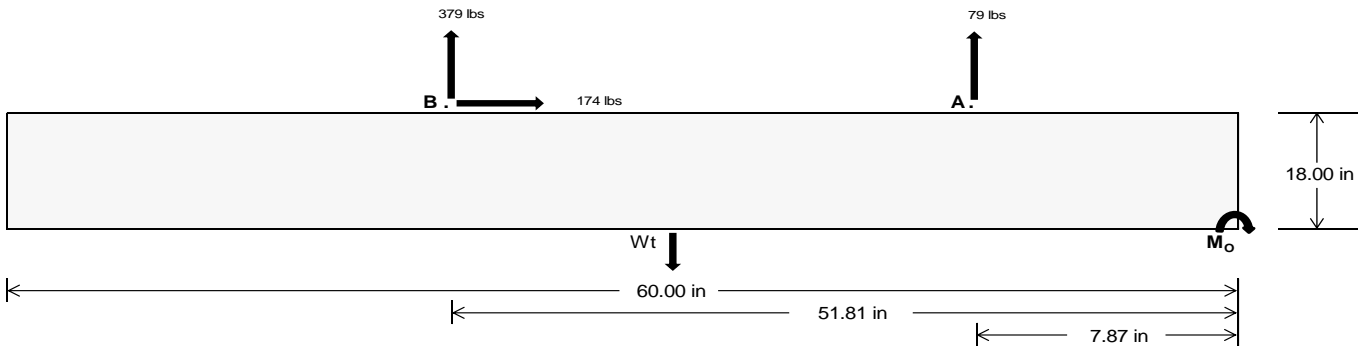
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	350.94	1645.78	k
Compressive Load =	1627.09	1270.92	k
Lateral Load =	33.34	754.83	k
Moment (Weak Axis) =	0.05	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23376.9$ in-lbs
Resisting Force Required = 779.23 lbs
S.F. = 1.67
Weight Required = 1298.72 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 174.11 lbs
Friction = 0.4
Weight Required = 435.26 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 174.11 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	574 lbs	574 lbs	574 lbs	574 lbs	527 lbs	527 lbs	527 lbs	527 lbs	783 lbs	783 lbs	783 lbs	783 lbs	-159 lbs	-159 lbs	-159 lbs	-159 lbs
F_B	419 lbs	419 lbs	419 lbs	419 lbs	465 lbs	465 lbs	465 lbs	465 lbs	630 lbs	630 lbs	630 lbs	630 lbs	-757 lbs	-757 lbs	-757 lbs	-757 lbs
F_V	49 lbs	49 lbs	49 lbs	49 lbs	311 lbs	311 lbs	311 lbs	311 lbs	266 lbs	266 lbs	266 lbs	266 lbs	-348 lbs	-348 lbs	-348 lbs	-348 lbs
P_{total}	2986 lbs	3077 lbs	3168 lbs	3258 lbs	2985 lbs	3076 lbs	3167 lbs	3257 lbs	3407 lbs	3497 lbs	3588 lbs	3679 lbs	280 lbs	335 lbs	389 lbs	443 lbs
M	371 lbs-ft	371 lbs-ft	371 lbs-ft	371 lbs-ft	594 lbs-ft	594 lbs-ft	594 lbs-ft	594 lbs-ft	698 lbs-ft	698 lbs-ft	698 lbs-ft	698 lbs-ft	562 lbs-ft	562 lbs-ft	562 lbs-ft	562 lbs-ft
e	0.12 ft	0.12 ft	0.12 ft	0.11 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.00 ft	1.68 ft	1.44 ft	1.27 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	277.2 psf	274.6 psf	272.2 psf	270.0 psf	248.0 psf	246.7 psf	245.4 psf	244.3 psf	280.2 psf	277.5 psf	275.0 psf	272.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	374.4 psf	367.6 psf	361.3 psf	355.6 psf	403.4 psf	395.3 psf	387.9 psf	381.1 psf	463.1 psf	452.4 psf	442.6 psf	433.6 psf	205.3 psf	141.6 psf	122.7 psf	115.0 psf

Maximum Bearing Pressure = 463 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

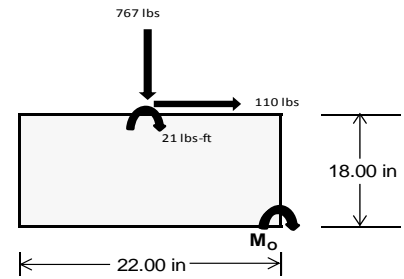
Overturning Check

$M_o = 516.9 \text{ ft-lbs}$
 Resisting Force Required = 563.91 lbs
 S.F. = 1.67
 Weight Required = 939.85 lbs
 Minimum Width = 22 in in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	122 lbs	130 lbs	69 lbs	308 lbs	767 lbs	267 lbs	73 lbs	-2 lbs	23 lbs
F_v	18 lbs	146 lbs	18 lbs	12 lbs	110 lbs	14 lbs	18 lbs	145 lbs	18 lbs
P_{total}	2591 lbs	2598 lbs	2538 lbs	2657 lbs	3116 lbs	2617 lbs	795 lbs	720 lbs	745 lbs
M	52 lbs-ft	245 lbs-ft	55 lbs-ft	36 lbs-ft	186 lbs-ft	42 lbs-ft	52 lbs-ft	245 lbs-ft	54 lbs-ft
e	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.07 ft	0.34 ft	0.07 ft
$L/6$	0.31 ft	1.64 ft	1.79 ft	1.81 ft	1.71 ft	1.80 ft	1.70 ft	1.15 ft	1.69 ft
f_{min}	264.0 sqft	195.8 sqft	257.4 sqft	277.2 sqft	273.6 sqft	270.4 sqft	68.0 sqft	-9.0 sqft	61.8 sqft
f_{max}	301.2 psf	371.1 psf	296.3 psf	302.6 psf	406.3 psf	300.5 psf	105.4 psf	166.1 psf	100.7 psf



Maximum Bearing Pressure = 406 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.525 k
Allowable Uplift =	1.214 k
Utilization =	<u>43%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.115 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.252 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>22%</u>

Diagonal Strut

Maximum Axial Load =	0.340 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.188 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.207 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.086 in
	<u>0.086 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 72.00 \text{ in}$$

$$J = 0.255$$

$$187.484$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.9 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 72.00 \text{ in}$$

$$J = 0.255$$

$$194.691$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.8$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.230 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.09 \\ &23.5807 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.4 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.09 \\ &24.5845 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.4 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.442 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.41804$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77853$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 13.5508 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 13.55 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 6.80 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	308.85	1	.076	2	.466	1	0	10	0	1	0	15
30			min	-364.387	3	.014	15	-.381	5	0	4	0	3	0	6
31		16	max	308.957	1	.044	2	.466	1	0	10	.001	1	0	15
32			min	-364.307	3	-.005	3	-.477	5	0	4	0	3	0	6
33		17	max	309.063	1	.012	2	.466	1	0	10	.001	1	0	15
34			min	-364.227	3	-.029	3	-.573	5	0	4	0	3	0	6
35		18	max	309.17	1	-.015	15	.466	1	0	10	.001	1	0	15
36			min	-364.147	3	-.06	4	-.67	5	0	4	0	3	0	6
37		19	max	309.277	1	-.025	15	.466	1	0	10	.001	1	0	15
38			min	-364.067	3	-.101	4	-.766	5	0	4	0	3	0	6
39	M3	1	max	79.9	2	1.794	6	-.027	12	0	5	.001	1	0	6
40			min	-86.219	3	.421	15	-1.398	4	0	1	0	12	0	15
41		2	max	79.832	2	1.616	6	-.027	12	0	5	.001	1	0	6
42			min	-86.27	3	.379	15	-1.264	4	0	1	0	12	0	15
43		3	max	79.764	2	1.439	6	-.027	12	0	5	.001	1	0	2
44			min	-86.321	3	.337	15	-1.13	4	0	1	0	15	0	3
45		4	max	79.696	2	1.261	6	-.027	12	0	5	0	1	0	15
46			min	-86.372	3	.296	15	-.997	4	0	1	0	5	0	4
47		5	max	79.629	2	1.083	6	-.027	12	0	5	0	1	0	15
48			min	-86.423	3	.254	15	-.863	4	0	1	0	5	0	4
49		6	max	79.561	2	.906	6	-.027	12	0	5	0	1	0	15
50			min	-86.474	3	.212	15	-.73	4	0	1	0	5	0	4
51		7	max	79.493	2	.728	6	-.027	12	0	5	0	1	0	15
52			min	-86.525	3	.17	15	-.596	4	0	1	0	5	0	4
53		8	max	79.425	2	.551	6	-.027	12	0	5	0	1	0	15
54			min	-86.576	3	.129	15	-.462	4	0	1	0	5	-.001	4
55		9	max	79.357	2	.373	6	-.027	12	0	5	0	1	0	15
56			min	-86.627	3	.087	15	-.396	1	0	1	0	5	-.001	4
57		10	max	79.289	2	.195	6	-.027	12	0	5	0	1	0	15
58			min	-86.677	3	.045	15	-.396	1	0	1	0	5	-.001	4
59		11	max	79.221	2	.033	2	.019	5	0	5	0	1	0	15
60			min	-86.728	3	-.003	3	-.396	1	0	1	0	5	-.001	4
61		12	max	79.154	2	-.038	15	.153	5	0	5	0	1	0	15
62			min	-86.779	3	-.16	4	-.396	1	0	1	0	5	-.001	4
63		13	max	79.086	2	-.08	15	.287	5	0	5	0	1	0	15
64			min	-86.83	3	-.338	4	-.396	1	0	1	0	5	-.001	4
65		14	max	79.018	2	-.122	15	.42	5	0	5	0	1	0	15
66			min	-86.881	3	-.516	4	-.396	1	0	1	0	5	-.001	4
67		15	max	78.95	2	-.164	15	.554	5	0	5	0	1	0	15
68			min	-86.932	3	-.693	4	-.396	1	0	1	0	5	0	4
69		16	max	78.882	2	-.205	15	.688	5	0	5	0	12	0	15
70			min	-86.983	3	-.871	4	-.396	1	0	1	0	4	0	4
71		17	max	78.814	2	-.247	15	.821	5	0	5	0	12	0	15
72			min	-87.034	3	-1.048	4	-.396	1	0	1	0	4	0	4
73		18	max	78.746	2	-.289	15	.955	5	0	5	0	12	0	15
74			min	-87.085	3	-1.226	4	-.396	1	0	1	0	1	0	4
75		19	max	78.679	2	-.331	15	1.088	5	0	5	0	5	0	1
76			min	-87.135	3	-1.404	4	-.396	1	0	1	0	1	0	1
77	M4	1	max	431.447	1	0	1	-.093	10	0	1	0	5	0	1
78			min	-75.59	3	0	1	-24.646	4	0	1	0	1	0	1
79		2	max	431.511	1	0	1	-.093	10	0	1	0	12	0	1
80			min	-75.542	3	0	1	-24.702	4	0	1	-.002	4	0	1
81		3	max	431.576	1	0	1	-.093	10	0	1	0	12	0	1
82			min	-75.493	3	0	1	-24.758	4	0	1	-.004	4	0	1
83		4	max	431.641	1	0	1	-.093	10	0	1	0	12	0	1
84			min	-75.445	3	0	1	-24.814	4	0	1	-.007	4	0	1
85		5	max	431.705	1	0	1	-.093	10	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-75.396	3	0	1	-24.87	4	0	1	-.009	4	0	1
87		6	max	431.77	1	0	1	-.093	10	0	1	0	12	0	1
88			min	-75.348	3	0	1	-24.926	4	0	1	-.011	4	0	1
89		7	max	431.835	1	0	1	-.093	10	0	1	0	12	0	1
90			min	-75.299	3	0	1	-24.982	4	0	1	-.013	4	0	1
91		8	max	431.9	1	0	1	-.093	10	0	1	0	10	0	1
92			min	-75.251	3	0	1	-25.038	4	0	1	-.016	4	0	1
93		9	max	431.964	1	0	1	-.093	10	0	1	0	10	0	1
94			min	-75.202	3	0	1	-25.095	4	0	1	-.018	4	0	1
95		10	max	432.029	1	0	1	-.093	10	0	1	0	10	0	1
96			min	-75.154	3	0	1	-25.151	4	0	1	-.02	4	0	1
97		11	max	432.094	1	0	1	-.093	10	0	1	0	10	0	1
98			min	-75.105	3	0	1	-25.207	4	0	1	-.022	4	0	1
99		12	max	432.158	1	0	1	-.093	10	0	1	0	10	0	1
100			min	-75.057	3	0	1	-25.263	4	0	1	-.025	4	0	1
101		13	max	432.223	1	0	1	-.093	10	0	1	0	10	0	1
102			min	-75.008	3	0	1	-25.319	4	0	1	-.027	4	0	1
103		14	max	432.288	1	0	1	-.093	10	0	1	0	10	0	1
104			min	-74.96	3	0	1	-25.375	4	0	1	-.029	4	0	1
105		15	max	432.353	1	0	1	-.093	10	0	1	0	10	0	1
106			min	-74.911	3	0	1	-25.431	4	0	1	-.031	4	0	1
107		16	max	432.417	1	0	1	-.093	10	0	1	0	10	0	1
108			min	-74.862	3	0	1	-25.487	4	0	1	-.034	4	0	1
109		17	max	432.482	1	0	1	-.093	10	0	1	0	10	0	1
110			min	-74.814	3	0	1	-25.543	4	0	1	-.036	4	0	1
111		18	max	432.547	1	0	1	-.093	10	0	1	0	10	0	1
112			min	-74.765	3	0	1	-25.599	4	0	1	-.038	4	0	1
113		19	max	432.611	1	0	1	-.093	10	0	1	0	10	0	1
114			min	-74.717	3	0	1	-25.655	4	0	1	-.04	4	0	1
115	M6	1	max	994.863	1	.63	6	1.025	4	0	3	0	3	0	1
116			min	-1188.472	3	.144	15	-.18	3	0	5	0	9	0	1
117		2	max	994.969	1	.589	6	.928	4	0	3	0	4	0	15
118			min	-1188.392	3	.135	15	-.18	3	0	5	0	9	0	6
119		3	max	995.076	1	.548	6	.832	4	0	3	0	4	0	15
120			min	-1188.312	3	.125	15	-.18	3	0	5	0	10	0	6
121		4	max	995.182	1	.506	6	.735	4	0	3	0	4	0	15
122			min	-1188.232	3	.115	15	-.18	3	0	5	0	10	0	6
123		5	max	995.289	1	.465	6	.639	4	0	3	0	4	0	15
124			min	-1188.152	3	.105	15	-.18	3	0	5	0	3	0	6
125		6	max	995.395	1	.424	6	.543	4	0	3	0	4	0	15
126			min	-1188.072	3	.096	15	-.18	3	0	5	0	3	0	6
127		7	max	995.502	1	.385	2	.446	4	0	3	0	4	0	15
128			min	-1187.993	3	.086	15	-.18	3	0	5	0	3	0	6
129		8	max	995.609	1	.353	2	.35	4	0	3	0	4	0	15
130			min	-1187.913	3	.076	15	-.18	3	0	5	0	3	0	6
131		9	max	995.715	1	.321	2	.253	4	0	3	0	4	0	15
132			min	-1187.833	3	.067	15	-.18	3	0	5	0	3	0	6
133		10	max	995.822	1	.288	2	.18	1	0	3	0	4	0	15
134			min	-1187.753	3	.057	15	-.18	3	0	5	0	3	0	6
135		11	max	995.928	1	.256	2	.18	1	0	3	0	4	0	15
136			min	-1187.673	3	.047	15	-.18	3	0	5	0	3	0	6
137		12	max	996.035	1	.224	2	.18	1	0	3	0	4	0	15
138			min	-1187.593	3	.036	12	-.18	3	0	5	0	3	0	6
139		13	max	996.141	1	.192	2	.18	1	0	3	0	4	0	15
140			min	-1187.513	3	.02	12	-.18	3	0	5	0	3	0	2
141		14	max	996.248	1	.16	2	.18	1	0	3	0	4	0	15
142			min	-1187.433	3	0	3	-.277	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	996.354	1	.128	2	.18	1	0	3	0	4	0	15
144		min	-1187.353	3	-.024	3	-.373	5	0	5	0	3	0	2
145	16	max	996.461	1	.096	2	.18	1	0	3	0	4	0	15
146		min	-1187.273	3	-.048	3	-.469	5	0	5	0	3	0	2
147	17	max	996.567	1	.063	2	.18	1	0	3	0	4	0	15
148		min	-1187.193	3	-.072	3	-.566	5	0	5	0	3	0	2
149	18	max	996.674	1	.031	2	.18	1	0	3	0	4	0	15
150		min	-1187.114	3	-.096	3	-.662	5	0	5	0	3	0	2
151	19	max	996.781	1	0	2	.18	1	0	3	0	1	0	15
152		min	-1187.034	3	-.121	3	-.759	5	0	5	0	3	0	2
153	M7	1	max	340.368	2	1.803	.008	4	0	1	0	4	0	2
154		min	-256.856	3	.428	15	-1.402	5	0	3	0	3	0	12
155	2	max	340.3	2	1.625	4	.008	1	0	1	0	4	0	2
156		min	-256.906	3	.386	15	-1.268	5	0	3	0	3	0	3
157	3	max	340.232	2	1.448	4	.008	1	0	1	0	4	0	2
158		min	-256.957	3	.344	15	-1.135	5	0	3	0	3	0	3
159	4	max	340.164	2	1.27	4	.008	1	0	1	0	1	0	2
160		min	-257.008	3	.302	15	-1.001	5	0	3	0	3	0	3
161	5	max	340.096	2	1.092	4	.008	1	0	1	0	1	0	15
162		min	-257.059	3	.261	15	-.867	5	0	3	0	5	0	6
163	6	max	340.029	2	.915	4	.008	1	0	1	0	1	0	15
164		min	-257.11	3	.219	15	-.734	5	0	3	0	5	0	6
165	7	max	339.961	2	.737	4	.008	1	0	1	0	1	0	15
166		min	-257.161	3	.177	15	-.6	5	0	3	0	5	0	6
167	8	max	339.893	2	.559	4	.008	1	0	1	0	1	0	15
168		min	-257.212	3	.135	15	-.466	5	0	3	0	5	0	6
169	9	max	339.825	2	.382	4	.008	1	0	1	0	1	0	15
170		min	-257.263	3	.094	15	-.333	5	0	3	0	5	-.001	6
171	10	max	339.757	2	.218	2	.008	1	0	1	0	1	0	15
172		min	-257.314	3	.044	12	-.199	5	0	3	0	5	-.001	6
173	11	max	339.689	2	.079	2	.008	1	0	1	0	1	0	15
174		min	-257.364	3	-.043	3	-.066	5	0	3	0	5	-.001	6
175	12	max	339.621	2	-.032	15	.069	4	0	1	0	1	0	15
176		min	-257.415	3	-.151	6	-.009	2	0	3	0	5	-.001	6
177	13	max	339.554	2	-.073	15	.202	4	0	1	0	1	0	15
178		min	-257.466	3	-.329	6	-.009	2	0	3	0	5	-.001	6
179	14	max	339.486	2	-.115	15	.336	4	0	1	0	1	0	15
180		min	-257.517	3	-.507	6	-.009	2	0	3	0	5	-.001	6
181	15	max	339.418	2	-.157	15	.469	4	0	1	0	1	0	15
182		min	-257.568	3	-.684	6	-.009	2	0	3	0	5	0	6
183	16	max	339.35	2	-.199	15	.603	4	0	1	0	1	0	15
184		min	-257.619	3	-.862	6	-.009	2	0	3	0	5	0	6
185	17	max	339.282	2	-.24	15	.737	4	0	1	0	1	0	15
186		min	-257.67	3	-1.04	6	-.009	2	0	3	0	5	0	6
187	18	max	339.214	2	-.282	15	.87	4	0	1	0	1	0	15
188		min	-257.721	3	-1.217	6	-.009	2	0	3	0	5	0	6
189	19	max	339.146	2	-.324	15	1.004	4	0	1	0	1	0	1
190		min	-257.772	3	-1.395	6	-.009	2	0	3	0	3	0	1
191	M8	1	max	1250.442	1	0	.604	1	0	1	0	4	0	1
192		min	-270.824	3	0	1	-24.947	4	0	1	0	1	0	1
193	2	max	1250.507	1	0	1	.604	1	0	1	0	1	0	1
194		min	-270.775	3	0	1	-25.003	4	0	1	-.002	4	0	1
195	3	max	1250.572	1	0	1	.604	1	0	1	0	1	0	1
196		min	-270.727	3	0	1	-25.06	4	0	1	-.004	4	0	1
197	4	max	1250.637	1	0	1	.604	1	0	1	0	1	0	1
198		min	-270.678	3	0	1	-25.116	4	0	1	-.007	4	0	1
199	5	max	1250.701	1	0	1	.604	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-270.63	3	0	1	-25.172	4	0	1	-.009	4	0	1
201		6	max	1250.766	1	0	1	.604	1	0	1	0	1	0	1
202			min	-270.581	3	0	1	-25.228	4	0	1	-.011	4	0	1
203		7	max	1250.831	1	0	1	.604	1	0	1	0	1	0	1
204			min	-270.533	3	0	1	-25.284	4	0	1	-.013	4	0	1
205		8	max	1250.895	1	0	1	.604	1	0	1	0	1	0	1
206			min	-270.484	3	0	1	-25.34	4	0	1	-.016	4	0	1
207		9	max	1250.96	1	0	1	.604	1	0	1	0	1	0	1
208			min	-270.436	3	0	1	-25.396	4	0	1	-.018	4	0	1
209		10	max	1251.025	1	0	1	.604	1	0	1	0	1	0	1
210			min	-270.387	3	0	1	-25.452	4	0	1	-.02	4	0	1
211		11	max	1251.09	1	0	1	.604	1	0	1	0	1	0	1
212			min	-270.338	3	0	1	-25.508	4	0	1	-.023	4	0	1
213		12	max	1251.154	1	0	1	.604	1	0	1	0	1	0	1
214			min	-270.29	3	0	1	-25.564	4	0	1	-.025	4	0	1
215		13	max	1251.219	1	0	1	.604	1	0	1	0	1	0	1
216			min	-270.241	3	0	1	-25.62	4	0	1	-.027	4	0	1
217		14	max	1251.284	1	0	1	.604	1	0	1	0	1	0	1
218			min	-270.193	3	0	1	-25.676	4	0	1	-.029	4	0	1
219		15	max	1251.348	1	0	1	.604	1	0	1	0	1	0	1
220			min	-270.144	3	0	1	-25.733	4	0	1	-.032	4	0	1
221		16	max	1251.413	1	0	1	.604	1	0	1	0	1	0	1
222			min	-270.096	3	0	1	-25.789	4	0	1	-.034	4	0	1
223		17	max	1251.478	1	0	1	.604	1	0	1	0	1	0	1
224			min	-270.047	3	0	1	-25.845	4	0	1	-.036	4	0	1
225		18	max	1251.542	1	0	1	.604	1	0	1	0	1	0	1
226			min	-269.999	3	0	1	-25.901	4	0	1	-.039	4	0	1
227		19	max	1251.607	1	0	1	.604	1	0	1	0	1	0	1
228			min	-269.95	3	0	1	-25.957	4	0	1	-.041	4	0	1
229	M10	1	max	309.947	1	.669	4	1.196	5	0	1	0	1	0	1
230			min	-344.211	3	.168	15	-.107	1	-.002	5	0	3	0	1
231		2	max	310.053	1	.628	4	1.099	5	0	1	0	4	0	15
232			min	-344.131	3	.158	15	-.107	1	-.002	5	0	3	0	4
233		3	max	310.16	1	.586	4	1.003	5	0	1	0	4	0	15
234			min	-344.051	3	.149	15	-.107	1	-.002	5	0	3	0	4
235		4	max	310.266	1	.545	4	.906	5	0	1	0	4	0	15
236			min	-343.971	3	.139	15	-.107	1	-.002	5	0	3	0	4
237		5	max	310.373	1	.504	4	.81	5	0	1	0	4	0	15
238			min	-343.891	3	.129	15	-.107	1	-.002	5	0	3	0	4
239		6	max	310.479	1	.462	4	.713	5	0	1	0	4	0	15
240			min	-343.811	3	.12	15	-.107	1	-.002	5	0	3	0	4
241		7	max	310.586	1	.421	4	.617	5	0	1	0	4	0	15
242			min	-343.731	3	.11	15	-.107	1	-.002	5	0	3	0	4
243		8	max	310.692	1	.38	4	.52	5	0	1	0	4	0	15
244			min	-343.651	3	.1	15	-.107	1	-.002	5	0	3	0	4
245		9	max	310.799	1	.339	4	.424	5	0	1	.001	4	0	15
246			min	-343.572	3	.09	15	-.107	1	-.002	5	0	3	0	4
247		10	max	310.905	1	.297	4	.328	5	0	1	.001	4	0	15
248			min	-343.492	3	.081	15	-.107	1	-.002	5	0	3	0	4
249		11	max	311.012	1	.256	4	.231	5	0	1	.001	4	0	15
250			min	-343.412	3	.071	15	-.107	1	-.002	5	0	3	0	4
251		12	max	311.118	1	.215	4	.135	5	0	1	.001	4	0	15
252			min	-343.332	3	.061	15	-.107	1	-.002	5	0	3	0	4
253		13	max	311.225	1	.174	4	.038	5	0	1	.001	4	0	15
254			min	-343.252	3	.052	15	-.107	1	-.002	5	0	3	0	4
255		14	max	311.332	1	.132	4	-.023	12	0	1	.001	5	0	15
256			min	-343.172	3	.037	9	-.107	1	-.002	5	0	3	0	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	311.438	1	.091	4	-.023	12	0	1	.001	5	0	15
258			min	-343.092	3	.01	9	-.164	4	-.002	5	0	3	0	4
259		16	max	311.545	1	.05	4	-.023	12	0	1	.001	5	0	15
260			min	-343.012	3	-.016	9	-.261	4	-.002	5	0	3	0	4
261		17	max	311.651	1	.019	5	-.023	12	0	1	.001	5	0	15
262			min	-342.932	3	-.043	9	-.357	4	-.002	5	0	3	0	4
263		18	max	311.758	1	.004	5	-.023	12	0	1	.001	5	0	15
264			min	-342.852	3	-.07	9	-.454	4	-.002	5	0	3	0	4
265		19	max	311.864	1	-.007	15	-.023	12	0	1	0	5	0	15
266			min	-342.772	3	-.097	9	-.55	4	-.002	5	0	1	0	4
267	M11	1	max	79.543	2	1.79	6	.45	1	.001	4	.001	5	0	6
268			min	-86.826	3	.418	15	-1.193	5	0	10	-.001	1	0	15
269		2	max	79.475	2	1.613	6	.45	1	.001	4	0	5	0	6
270			min	-86.877	3	.377	15	-1.059	5	0	10	-.001	1	0	15
271		3	max	79.407	2	1.435	6	.45	1	.001	4	0	5	0	2
272			min	-86.928	3	.335	15	-.925	5	0	10	-.001	1	0	3
273		4	max	79.339	2	1.258	6	.45	1	.001	4	0	5	0	15
274			min	-86.979	3	.293	15	-.792	5	0	10	0	1	0	4
275		5	max	79.271	2	1.08	6	.45	1	.001	4	0	5	0	15
276			min	-87.03	3	.251	15	-.658	5	0	10	0	1	0	4
277		6	max	79.204	2	.902	6	.45	1	.001	4	0	3	0	15
278			min	-87.081	3	.21	15	-.525	5	0	10	0	1	0	4
279		7	max	79.136	2	.725	6	.45	1	.001	4	0	3	0	15
280			min	-87.132	3	.168	15	-.391	5	0	10	0	1	0	4
281		8	max	79.068	2	.547	6	.45	1	.001	4	0	3	0	15
282			min	-87.182	3	.126	15	-.257	5	0	10	0	1	-.001	4
283		9	max	79	2	.369	6	.45	1	.001	4	0	3	0	15
284			min	-87.233	3	.084	15	-.124	5	0	10	0	1	-.001	4
285		10	max	78.932	2	.192	6	.45	1	.001	4	0	3	0	15
286			min	-87.284	3	.043	15	-.004	3	0	10	0	1	-.001	4
287		11	max	78.864	2	.033	2	.45	1	.001	4	0	3	0	15
288			min	-87.335	3	-.022	3	-.004	3	0	10	0	1	-.001	4
289		12	max	78.796	2	-.041	15	.45	1	.001	4	0	3	0	15
290			min	-87.386	3	-.164	4	-.004	3	0	10	0	1	-.001	4
291		13	max	78.729	2	-.083	15	.503	4	.001	4	0	3	0	15
292			min	-87.437	3	-.342	4	-.004	3	0	10	0	1	-.001	4
293		14	max	78.661	2	-.124	15	.636	4	.001	4	0	3	0	15
294			min	-87.488	3	-.519	4	-.004	3	0	10	0	10	-.001	4
295		15	max	78.593	2	-.166	15	.77	4	.001	4	0	4	0	15
296			min	-87.539	3	-.697	4	-.004	3	0	10	0	10	0	4
297		16	max	78.525	2	-.208	15	.904	4	.001	4	0	4	0	15
298			min	-87.59	3	-.875	4	-.004	3	0	10	0	10	0	4
299		17	max	78.457	2	-.25	15	1.037	4	.001	4	0	4	0	15
300			min	-87.64	3	-1.052	4	-.004	3	0	10	0	10	0	4
301		18	max	78.389	2	-.292	15	1.171	4	.001	4	.001	4	0	15
302			min	-87.691	3	-1.23	4	-.004	3	0	10	0	10	0	4
303		19	max	78.321	2	-.333	15	1.305	4	.001	4	.001	4	0	1
304			min	-87.742	3	-1.408	4	-.004	3	0	10	0	10	0	1
305	M12	1	max	431.302	1	0	1	2.133	1	0	1	0	4	0	1
306			min	-75.18	3	0	1	-22.831	5	0	1	0	3	0	1
307		2	max	431.367	1	0	1	2.133	1	0	1	0	1	0	1
308			min	-75.132	3	0	1	-22.888	5	0	1	-.002	5	0	1
309		3	max	431.431	1	0	1	2.133	1	0	1	0	1	0	1
310			min	-75.083	3	0	1	-22.944	5	0	1	-.004	5	0	1
311		4	max	431.496	1	0	1	2.133	1	0	1	0	1	0	1
312			min	-75.035	3	0	1	-.23	5	0	1	-.006	5	0	1
313		5	max	431.561	1	0	1	2.133	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-74.986	3	0	1	-23.056	5	0	1	-.008	5	0	1
315		6	max	431.625	1	0	1	2.133	1	0	1	0	1	0	1
316			min	-74.938	3	0	1	-23.112	5	0	1	-.01	5	0	1
317		7	max	431.69	1	0	1	2.133	1	0	1	.001	1	0	1
318			min	-74.889	3	0	1	-23.168	5	0	1	-.012	5	0	1
319		8	max	431.755	1	0	1	2.133	1	0	1	.001	1	0	1
320			min	-74.841	3	0	1	-23.224	5	0	1	-.014	5	0	1
321		9	max	431.819	1	0	1	2.133	1	0	1	.002	1	0	1
322			min	-74.792	3	0	1	-23.28	5	0	1	-.016	5	0	1
323		10	max	431.884	1	0	1	2.133	1	0	1	.002	1	0	1
324			min	-74.744	3	0	1	-23.336	5	0	1	-.019	5	0	1
325		11	max	431.949	1	0	1	2.133	1	0	1	.002	1	0	1
326			min	-74.695	3	0	1	-23.392	5	0	1	-.021	5	0	1
327		12	max	432.014	1	0	1	2.133	1	0	1	.002	1	0	1
328			min	-74.647	3	0	1	-23.448	5	0	1	-.023	5	0	1
329		13	max	432.078	1	0	1	2.133	1	0	1	.002	1	0	1
330			min	-74.598	3	0	1	-23.504	5	0	1	-.025	5	0	1
331		14	max	432.143	1	0	1	2.133	1	0	1	.003	1	0	1
332			min	-74.55	3	0	1	-23.56	5	0	1	-.027	5	0	1
333		15	max	432.208	1	0	1	2.133	1	0	1	.003	1	0	1
334			min	-74.501	3	0	1	-23.617	5	0	1	-.029	5	0	1
335		16	max	432.272	1	0	1	2.133	1	0	1	.003	1	0	1
336			min	-74.452	3	0	1	-23.673	5	0	1	-.031	5	0	1
337		17	max	432.337	1	0	1	2.133	1	0	1	.003	1	0	1
338			min	-74.404	3	0	1	-23.729	5	0	1	-.033	5	0	1
339		18	max	432.402	1	0	1	2.133	1	0	1	.003	1	0	1
340			min	-74.355	3	0	1	-23.785	5	0	1	-.035	5	0	1
341		19	max	432.467	1	0	1	2.133	1	0	1	.003	1	0	1
342			min	-74.307	3	0	1	-23.841	5	0	1	-.038	5	0	1
343	M1	1	max	99.89	1	345.038	3	-2.189	12	0	1	.083	1	0	1
344			min	4.277	12	-308.894	1	-42.119	1	0	3	.005	12	0	3
345		2	max	99.985	1	344.841	3	-2.189	12	0	1	.073	1	.067	1
346			min	4.325	12	-309.157	1	-42.119	1	0	3	.005	12	-.075	3
347		3	max	83.351	1	5.752	9	-2.238	12	0	3	.064	1	.133	1
348			min	2.562	10	-20.291	3	-41.863	1	0	1	.004	12	-.148	3
349		4	max	83.447	1	5.533	9	-2.238	12	0	3	.055	1	.134	1
350			min	2.642	10	-20.488	3	-41.863	1	0	1	.003	12	-.144	3
351		5	max	83.542	1	5.314	9	-2.238	12	0	3	.046	1	.134	1
352			min	2.721	10	-20.685	3	-41.863	1	0	1	.003	12	-.139	3
353		6	max	83.638	1	5.096	9	-2.238	12	0	3	.037	1	.135	1
354			min	2.801	10	-20.882	3	-41.863	1	0	1	.002	12	-.135	3
355		7	max	83.733	1	4.877	9	-2.238	12	0	3	.027	1	.135	1
356			min	2.881	10	-21.079	3	-41.863	1	0	1	.002	10	-.13	3
357		8	max	83.829	1	4.658	9	-2.238	12	0	3	.018	1	.136	1
358			min	2.96	10	-21.275	3	-41.863	1	0	1	.001	10	-.126	3
359		9	max	83.924	1	4.44	9	-2.238	12	0	3	.009	1	.137	1
360			min	3.04	10	-21.472	3	-41.863	1	0	1	0	10	-.121	3
361		10	max	84.02	1	4.221	9	-2.238	12	0	3	.002	4	.14	2
362			min	3.119	10	-21.669	3	-41.863	1	0	1	0	10	-.116	3
363		11	max	84.115	1	4.002	9	-2.238	12	0	3	0	3	.144	2
364			min	3.199	10	-21.866	3	-41.863	1	0	1	-.009	1	-.112	3
365		12	max	84.211	1	3.784	9	-2.238	12	0	3	0	12	.148	2
366			min	3.278	10	-22.063	3	-41.863	1	0	1	-.018	1	-.107	3
367		13	max	84.306	1	3.565	9	-2.238	12	0	3	0	12	.152	2
368			min	3.358	10	-22.259	3	-41.863	1	0	1	-.027	1	-.102	3
369		14	max	84.402	1	3.346	9	-2.238	12	0	3	-.001	12	.156	2
370			min	3.438	10	-22.456	3	-41.863	1	0	1	-.036	1	-.097	3





Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	1.29	15	-20.618	3	-26.909	5	0	10	-.044	1	-.139	3
429		6	max	83.767	1	5.073	9	40.804	1	0	1	.048	5	.135	1
430			min	1.319	15	-20.814	3	-26.667	5	0	10	-.035	1	-.135	3
431		7	max	83.862	1	4.854	9	40.804	1	0	1	.043	5	.135	1
432			min	1.348	15	-21.011	3	-26.425	5	0	10	-.027	1	-.13	3
433		8	max	83.958	1	4.635	9	40.804	1	0	1	.037	5	.136	1
434			min	1.376	15	-21.208	3	-26.183	5	0	10	-.018	1	-.126	3
435		9	max	84.053	1	4.417	9	40.804	1	0	1	.031	5	.137	1
436			min	1.405	15	-21.405	3	-25.941	5	0	10	-.009	1	-.121	3
437		10	max	84.149	1	4.198	9	40.804	1	0	1	.026	4	.14	2
438			min	1.434	15	-21.602	3	-25.699	5	0	10	0	1	-.116	3
439		11	max	84.244	1	3.979	9	40.804	1	0	1	.022	4	.144	2
440			min	1.463	15	-21.798	3	-25.457	5	0	10	0	10	-.112	3
441		12	max	84.34	1	3.761	9	40.804	1	0	1	.018	4	.148	2
442			min	1.492	15	-21.995	3	-25.215	5	0	10	.001	10	-.107	3
443		13	max	84.435	1	3.542	9	40.804	1	0	1	.026	1	.152	2
444			min	1.521	15	-22.192	3	-24.973	5	0	10	.002	10	-.102	3
445		14	max	84.531	1	3.323	9	40.804	1	0	1	.035	1	.156	2
446			min	1.549	15	-22.389	3	-24.731	5	0	10	.002	15	-.097	3
447		15	max	84.626	1	3.105	9	40.804	1	0	1	.044	1	.16	2
448			min	1.578	15	-22.586	3	-24.489	5	0	10	-.002	5	-.093	3
449		16	max	81.409	2	35.997	2	41.218	1	0	10	.054	1	.164	2
450			min	-31.711	3	-86.532	3	-23.039	5	0	4	-.005	5	-.087	3
451		17	max	81.505	2	35.734	2	41.218	1	0	10	.063	1	.156	2
452			min	-31.639	3	-86.729	3	-22.797	5	0	4	-.01	5	-.068	3
453		18	max	4.795	5	362.682	2	43.325	1	0	2	.072	1	.079	2
454			min	-99.604	1	-157.642	3	-45.344	5	0	3	-.02	5	-.034	3
455		19	max	4.84	5	362.42	2	43.325	1	0	2	.082	1	0	2
456			min	-99.508	1	-157.838	3	-45.102	5	0	3	-.03	5	0	3
457	M13	1	max	165.403	4	308.55	1	-1.302	15	0	1	.082	1	0	1
458			min	2.906	10	-345.016	3	-99.518	1	0	3	0	15	0	3
459		2	max	158.855	4	217.919	1	-.522	15	0	1	.023	1	.196	3
460			min	2.906	10	-243.549	3	-75.958	1	0	3	-.001	5	-.175	1
461		3	max	152.307	4	127.289	1	.281	5	0	1	.004	3	.325	3
462			min	2.906	10	-142.082	3	-52.397	1	0	3	-.02	1	-.291	1
463		4	max	145.76	4	36.658	1	1.487	5	0	1	.001	3	.386	3
464			min	2.906	10	-40.616	3	-28.837	1	0	3	-.047	1	-.345	1
465		5	max	139.212	4	60.851	3	2.693	5	0	1	0	5	.379	3
466			min	2.906	10	-53.973	1	-5.277	1	0	3	-.058	1	-.339	1
467		6	max	132.664	4	162.318	3	18.284	1	0	1	.003	5	.304	3
468			min	2.906	10	-144.604	1	-.604	3	0	3	-.054	1	-.273	1
469		7	max	126.116	4	263.785	3	41.844	1	0	1	.006	5	.162	3
470			min	2.906	10	-235.235	1	.45	12	0	3	-.034	1	-.147	1
471		8	max	119.569	4	365.251	3	65.405	1	0	1	.01	4	.04	1
472			min	2.906	10	-325.865	1	1.206	12	0	3	0	3	-.047	3
473		9	max	113.021	4	466.718	3	88.965	1	0	1	.053	1	.288	1
474			min	2.906	10	-416.496	1	1.962	12	0	3	0	12	-.325	3
475		10	max	106.473	4	568.185	3	112.526	1	0	1	.121	1	.596	1
476			min	2.906	10	-507.127	1	2.719	12	0	3	.003	12	-.67	3
477		11	max	76.254	4	416.496	1	3.319	5	0	3	.053	1	.288	1
478			min	2.189	12	-466.718	3	-88.602	1	0	1	-.015	5	-.325	3
479		12	max	69.706	4	325.865	1	4.525	5	0	3	.002	2	.04	1
480			min	2.189	12	-365.251	3	-65.042	1	0	1	-.013	4	-.047	3
481		13	max	63.158	4	235.234	1	5.731	5	0	3	-.002	10	.162	3
482			min	2.189	12	-263.785	3	-41.481	1	0	1	-.034	1	-.147	1
483		14	max	56.611	4	144.604	1	6.937	5	0	3	-.003	12	.304	3
484			min	2.189	12	-162.318	3	-17.921	1	0	1	-.054	1	-.273	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	50.063	4	53.973	1	9.484	4	0	3	0	15	.379	3
486			min	2.189	12	-60.851	3	-.134	10	0	1	-.058	1	-.339	1
487		16	max	43.515	4	40.616	3	29.2	1	0	3	.006	5	.386	3
488			min	2.189	12	-36.658	1	2.009	12	0	1	-.046	1	-.345	1
489		17	max	42.218	1	142.083	3	52.761	1	0	3	.012	5	.325	3
490			min	2.189	12	-127.289	1	2.765	12	0	1	-.019	1	-.291	1
491		18	max	42.218	1	243.549	3	76.321	1	0	3	.025	4	.196	3
492			min	2.189	12	-217.919	1	3.521	12	0	1	0	10	-.175	1
493		19	max	42.218	1	345.016	3	99.881	1	0	3	.083	1	0	1
494			min	2.189	12	-308.55	1	4.277	12	0	1	.005	12	0	3
495	M16	1	max	45.091	5	362.566	2	4.84	5	0	3	.082	1	0	2
496			min	-43.221	1	-157.855	3	-99.517	1	0	2	-.03	5	0	3
497		2	max	38.543	5	256.059	2	6.046	5	0	3	.023	1	.09	3
498			min	-43.221	1	-111.64	3	-75.957	1	0	2	-.026	5	-.206	2
499		3	max	31.996	5	149.552	2	7.251	5	0	3	0	12	.149	3
500			min	-43.221	1	-65.425	3	-52.396	1	0	2	-.026	4	-.341	2
501		4	max	25.448	5	43.046	2	8.457	5	0	3	-.002	12	.177	3
502			min	-43.221	1	-19.211	3	-28.836	1	0	2	-.047	1	-.406	2
503		5	max	18.9	5	27.004	3	9.663	5	0	3	-.002	12	.174	3
504			min	-43.221	1	-63.461	2	-5.275	1	0	2	-.058	1	-.399	2
505		6	max	12.352	5	73.219	3	18.285	1	0	3	-.002	15	.141	3
506			min	-43.221	1	-169.968	2	-.134	3	0	2	-.054	1	-.321	2
507		7	max	5.805	5	119.434	3	41.846	1	0	3	.004	5	.077	3
508			min	-43.221	1	-276.475	2	.733	12	0	2	-.034	1	-.172	2
509		8	max	.298	3	165.648	3	65.406	1	0	3	.013	4	.048	2
510			min	-43.221	1	-382.982	2	1.489	12	0	2	-.003	3	-.018	3
511		9	max	.298	3	211.863	3	88.966	1	0	3	.053	1	.338	2
512			min	-43.221	1	-489.488	2	2.245	12	0	2	-.001	3	-.144	3
513		10	max	26.13	5	-11.979	15	112.527	1	0	14	.121	1	.7	2
514			min	-43.221	1	-595.995	2	-4.83	3	0	2	.003	12	-.301	3
515		11	max	19.583	5	489.488	2	3.122	5	0	2	.053	1	.338	2
516			min	-43.064	1	-211.863	3	-88.609	1	0	3	-.013	5	-.144	3
517		12	max	13.035	5	382.982	2	4.328	5	0	2	.002	2	.048	2
518			min	-43.064	1	-165.648	3	-65.048	1	0	3	-.011	4	-.018	3
519		13	max	6.487	5	276.475	2	5.534	5	0	2	-.001	12	.077	3
520			min	-43.064	1	-119.434	3	-41.488	1	0	3	-.034	1	-.172	2
521		14	max	.018	15	169.968	2	6.739	5	0	2	-.002	12	.141	3
522			min	-43.064	1	-73.219	3	-17.927	1	0	3	-.054	1	-.321	2
523		15	max	-2.397	12	63.461	2	9.263	4	0	2	.001	5	.174	3
524			min	-43.064	1	-27.004	3	-.133	10	0	3	-.058	1	-.399	2
525		16	max	-2.397	12	19.211	3	29.194	1	0	2	.007	5	.177	3
526			min	-43.064	1	-43.046	2	1.282	12	0	3	-.046	1	-.406	2
527		17	max	-2.397	12	65.425	3	52.754	1	0	2	.014	5	.149	3
528			min	-43.064	1	-149.552	2	2.038	12	0	3	-.019	1	-.341	2
529		18	max	-2.397	12	111.64	3	76.314	1	0	2	.026	4	.09	3
530			min	-43.064	1	-256.059	2	2.794	12	0	3	0	10	-.206	2
531		19	max	-2.397	12	157.855	3	99.875	1	0	2	.083	1	0	2
532			min	-43.064	1	-362.566	2	3.551	12	0	3	.004	12	0	3
533	M15	1	max	0	1	1.362	9	.057	3	0	9	0	9	0	1
534			min	-47.837	3	0	1	-.025	9	0	3	0	3	0	1
535		2	max	0	1	1.211	9	.057	3	0	9	0	9	0	1
536			min	-47.897	3	0	1	-.025	9	0	3	0	3	0	9
537		3	max	0	1	1.059	9	.057	3	0	9	0	9	0	1
538			min	-47.957	3	0	1	-.025	9	0	3	0	3	0	9
539		4	max	0	1	.908	9	.057	3	0	9	0	9	0	1
540			min	-48.016	3	0	1	-.025	9	0	3	0	3	-.001	9
541		5	max	0	1	.757	9	.057	3	0	9	0	9	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-48.076	3	0	1	-.025	9	0	3	0	3	-.002	9
543		6	max	0	1	.605	9	.057	3	0	9	0	9	0	1
544			min	-48.136	3	0	1	-.025	9	0	3	0	3	-.002	9
545		7	max	0	1	.454	9	.057	3	0	9	0	3	0	1
546			min	-48.195	3	0	1	-.025	9	0	3	0	9	-.002	9
547		8	max	0	1	.303	9	.057	3	0	9	0	3	0	1
548			min	-48.255	3	0	1	-.025	9	0	3	0	9	-.002	9
549		9	max	0	1	.151	9	.057	3	0	9	0	3	0	1
550			min	-48.315	3	0	1	-.025	9	0	3	0	9	-.002	9
551		10	max	0	1	0	1	.057	3	0	9	0	3	0	1
552			min	-48.374	3	0	1	-.025	9	0	3	0	9	-.002	9
553		11	max	0	1	0	1	.057	3	0	9	0	3	0	1
554			min	-48.434	3	-.151	9	-.025	9	0	3	0	9	-.002	9
555		12	max	0	1	0	1	.057	3	0	9	0	3	0	1
556			min	-48.494	3	-.303	9	-.025	9	0	3	0	9	-.002	9
557		13	max	0	1	0	1	.057	3	0	9	0	3	0	1
558			min	-48.553	3	-.454	9	-.025	9	0	3	0	9	-.002	9
559		14	max	0	1	0	1	.057	3	0	9	0	3	0	1
560			min	-48.613	3	-.605	9	-.025	9	0	3	0	9	-.002	9
561		15	max	0	1	0	1	.057	3	0	9	0	3	0	1
562			min	-48.673	3	-.757	9	-.025	9	0	3	0	9	-.002	9
563		16	max	0	1	0	1	.057	3	0	9	0	3	0	1
564			min	-48.732	3	-.908	9	-.025	9	0	3	0	9	-.001	9
565		17	max	0	1	0	1	.057	3	0	9	0	3	0	1
566			min	-48.792	3	-1.059	9	-.025	9	0	3	0	9	0	9
567		18	max	0	1	0	1	.057	3	0	9	0	3	0	1
568			min	-48.852	3	-1.211	9	-.025	9	0	3	0	9	0	9
569		19	max	0	1	0	1	.057	3	0	9	0	3	0	1
570			min	-48.911	3	-1.362	9	-.025	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.713	4	.239	4	0	3	0	3	0	1
572			min	-206.419	4	0	10	-.022	3	0	1	0	4	0	1
573		2	max	0	10	2.411	4	.216	4	0	3	0	3	0	10
574			min	-206.472	4	0	10	-.022	3	0	1	0	4	0	4
575		3	max	0	10	2.11	4	.193	4	0	3	0	3	0	10
576			min	-206.526	4	0	10	-.022	3	0	1	0	4	-.002	4
577		4	max	0	10	1.808	4	.171	4	0	3	0	3	0	10
578			min	-206.58	4	0	10	-.022	3	0	1	0	4	-.002	4
579		5	max	0	10	1.507	4	.148	4	0	3	0	3	0	10
580			min	-206.634	4	0	10	-.022	3	0	1	0	1	-.003	4
581		6	max	0	10	1.206	4	.125	4	0	3	0	3	0	10
582			min	-206.688	4	0	10	-.022	3	0	1	0	1	-.004	4
583		7	max	0	10	.904	4	.102	4	0	3	0	5	0	10
584			min	-206.742	4	0	10	-.022	3	0	1	0	1	-.004	4
585		8	max	0	10	.603	4	.08	4	0	3	0	5	0	10
586			min	-206.796	4	0	10	-.022	3	0	1	0	1	-.004	4
587		9	max	0	10	.301	4	.057	4	0	3	0	5	0	10
588			min	-206.85	4	0	10	-.022	3	0	1	0	1	-.004	4
589		10	max	0	10	0	1	.034	4	0	3	0	5	0	10
590			min	-206.904	4	0	1	-.022	3	0	1	0	1	-.004	4
591		11	max	0	10	0	10	.028	1	0	3	0	5	0	10
592			min	-206.958	4	-.301	4	-.022	3	0	1	0	1	-.004	4
593		12	max	0	10	0	10	.028	1	0	3	0	5	0	10
594			min	-207.011	4	-.603	4	-.022	3	0	1	0	1	-.004	4
595		13	max	0	10	0	10	.028	1	0	3	0	5	0	10
596			min	-207.065	4	-.904	4	-.038	5	0	1	0	3	-.004	4
597		14	max	0	10	0	10	.028	1	0	3	0	4	0	10
598			min	-207.119	4	-1.206	4	-.061	5	0	1	0	3	-.004	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.009	5	1.168e-4	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-8.512e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.013	5	1.361e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-8.581e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.018	5	1.555e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-8.649e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.022	4	1.748e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-8.718e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.027	4	1.942e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-8.787e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.031	4	2.135e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-8.856e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.035	4	2.328e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	1	-8.925e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.039	4	2.522e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-8.994e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.044	4	2.715e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-9.063e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.048	4	2.909e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-9.132e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.052	4	3.102e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-9.201e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.056	4	3.296e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-9.27e-4	5	NC	1	NC	1
67		15	max	0	3	.005	2	.06	4	3.489e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-9.339e-4	5	9729.974	2	NC	1
69		16	max	0	3	.006	2	.063	4	3.682e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-9.408e-4	5	8204.679	2	NC	1
71		17	max	0	3	.007	2	.067	4	3.876e-4	1	NC	3	NC	1
72			min	0	2	-.007	3	0	10	-9.476e-4	5	7032.09	2	NC	1
73		18	max	0	3	.008	2	.071	4	4.069e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	10	-9.545e-4	5	6119.823	2	NC	1
75		19	max	0	3	.009	2	.075	4	4.263e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	10	-9.614e-4	5	5403.397	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	3.858e-3	5	NC	1	NC	2
78			min	0	3	-.006	3	-.079	4	-5.298e-4	1	NC	1	244.708	4
79		2	max	.002	1	.007	2	0	10	3.858e-3	5	NC	1	NC	2
80			min	0	3	-.006	3	-.072	4	-5.298e-4	1	NC	1	266.759	4
81		3	max	.002	1	.007	2	0	10	3.858e-3	5	NC	1	NC	2
82			min	0	3	-.006	3	-.066	4	-5.298e-4	1	NC	1	293.004	4
83		4	max	.002	1	.007	2	0	10	3.858e-3	5	NC	1	NC	2
84			min	0	3	-.005	3	-.06	4	-5.298e-4	1	NC	1	324.548	4
85		5	max	.002	1	.006	2	0	10	3.858e-3	5	NC	1	NC	2
86			min	0	3	-.005	3	-.053	4	-5.298e-4	1	NC	1	362.895	4
87		6	max	.001	1	.006	2	0	10	3.858e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.047	4	-5.298e-4	1	NC	1	410.139	4
89		7	max	.001	1	.005	2	0	10	3.858e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.041	4	-5.298e-4	1	NC	1	469.259	4
91		8	max	.001	1	.005	2	0	10	3.858e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.035	4	-5.298e-4	1	NC	1	544.62	4
93		9	max	.001	1	.004	2	0	10	3.858e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.03	4	-5.298e-4	1	NC	1	642.823	4
95		10	max	.001	1	.004	2	0	10	3.858e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.025	4	-5.298e-4	1	NC	1	774.243	4
97		11	max	0	1	.003	2	0	10	3.858e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.02	4	-5.298e-4	1	NC	1	955.971	4
99		12	max	0	1	.003	2	0	10	3.858e-3	5	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.009	4	2.241e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-8.576e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.014	4	1.69e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-8.496e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.018	4	1.14e-5	1	NC	1	NC	1
162			min	0	2	-.007	3	0	1	-8.415e-4	4	8681.146	2	NC	1
163		6	max	0	3	.007	2	.023	4	1.319e-5	3	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	-8.334e-4	4	6958.11	2	NC	1
165		7	max	0	3	.008	2	.028	4	2.816e-5	3	NC	3	NC	1
166			min	-.001	2	-.01	3	0	1	-8.254e-4	4	5779.651	2	NC	1
167		8	max	.001	3	.009	2	.032	4	4.313e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	-.001	1	-8.173e-4	4	4915.621	2	NC	1
169		9	max	.001	3	.011	2	.037	4	5.81e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	-.001	1	-8.093e-4	4	4251.357	2	NC	1
171		10	max	.001	3	.012	2	.041	4	7.307e-5	3	NC	3	NC	1
172			min	-.002	2	-.014	3	-.001	1	-8.012e-4	4	3723.306	2	NC	1
173		11	max	.002	3	.014	2	.045	4	8.804e-5	3	NC	3	NC	1
174			min	-.002	2	-.015	3	-.001	1	-7.932e-4	4	3293.303	2	NC	1
175		12	max	.002	3	.016	2	.049	4	1.03e-4	3	NC	3	NC	1
176			min	-.002	2	-.016	3	-.002	1	-7.851e-4	4	2936.92	2	NC	1
177		13	max	.002	3	.017	2	.053	4	1.18e-4	3	NC	3	NC	1
178			min	-.003	2	-.017	3	-.002	1	-7.771e-4	4	2637.673	2	NC	1
179		14	max	.002	3	.019	2	.057	4	1.329e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	-.002	1	-7.69e-4	4	2383.938	2	NC	1
181		15	max	.002	3	.021	2	.061	4	1.479e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	-.002	1	-7.609e-4	4	2167.208	2	NC	1
183		16	max	.002	3	.023	2	.065	4	1.629e-4	3	NC	3	NC	1
184			min	-.003	2	-.02	3	-.002	1	-7.529e-4	4	1981.064	2	NC	1
185		17	max	.003	3	.025	2	.069	4	1.779e-4	3	NC	3	NC	1
186			min	-.003	2	-.021	3	-.002	1	-7.448e-4	4	1820.542	2	NC	1
187		18	max	.003	3	.027	2	.072	4	1.928e-4	3	NC	3	NC	1
188			min	-.004	2	-.022	3	-.002	1	-7.368e-4	4	1681.731	2	NC	1
189		19	max	.003	3	.029	2	.076	4	2.078e-4	3	NC	3	NC	1
190			min	-.004	2	-.023	3	-.002	1	-7.287e-4	4	1561.502	2	NC	1
191	M8	1	max	.006	1	.027	2	.002	1	3.653e-3	4	NC	1	NC	1
192			min	-.001	3	-.02	3	-.08	4	-1.617e-4	3	NC	1	241.83	4
193		2	max	.006	1	.026	2	.002	1	3.653e-3	4	NC	1	NC	1
194			min	-.001	3	-.019	3	-.073	4	-1.617e-4	3	NC	1	263.621	4
195		3	max	.005	1	.024	2	.002	1	3.653e-3	4	NC	1	NC	1
196			min	-.001	3	-.018	3	-.067	4	-1.617e-4	3	NC	1	289.556	4
197		4	max	.005	1	.023	2	.001	1	3.653e-3	4	NC	1	NC	1
198			min	-.001	3	-.017	3	-.06	4	-1.617e-4	3	NC	1	320.728	4
199		5	max	.005	1	.021	2	.001	1	3.653e-3	4	NC	1	NC	1
200			min	-.001	3	-.016	3	-.054	4	-1.617e-4	3	NC	1	358.624	4
201		6	max	.004	1	.02	2	.001	1	3.653e-3	4	NC	1	NC	1
202			min	0	3	-.014	3	-.048	4	-1.617e-4	3	NC	1	405.311	4
203		7	max	.004	1	.018	2	0	1	3.653e-3	4	NC	1	NC	1
204			min	0	3	-.013	3	-.042	4	-1.617e-4	3	NC	1	463.736	4
205		8	max	.004	1	.017	2	0	1	3.653e-3	4	NC	1	NC	1
206			min	0	3	-.012	3	-.036	4	-1.617e-4	3	NC	1	538.21	4
207		9	max	.003	1	.015	2	0	1	3.653e-3	4	NC	1	NC	1
208			min	0	3	-.011	3	-.03	4	-1.617e-4	3	NC	1	635.257	4
209		10	max	.003	1	.014	2	0	1	3.653e-3	4	NC	1	NC	1
210			min	0	3	-.01	3	-.025	4	-1.617e-4	3	NC	1	765.131	4
211		11	max	.003	1	.012	2	0	1	3.653e-3	4	NC	1	NC	1
212			min	0	3	-.009	3	-.02	4	-1.617e-4	3	NC	1	944.721	4
213		12	max	.002	1	.011	2	0	1	3.653e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.008	3	-.016	4	-1.617e-4	3	NC	1	1203.483	4
215		13	max	.002	1	.009	2	0	1	3.653e-3	4	NC	1	NC	1
216			min	0	3	-.007	3	-.012	4	-1.617e-4	3	NC	1	1596.674	4
217		14	max	.002	1	.008	2	0	1	3.653e-3	4	NC	1	NC	1
218			min	0	3	-.006	3	-.009	4	-1.617e-4	3	NC	1	2238.265	4
219		15	max	.001	1	.006	2	0	1	3.653e-3	4	NC	1	NC	1
220			min	0	3	-.004	3	-.006	4	-1.617e-4	3	NC	1	3396.016	4
221		16	max	0	1	.005	2	0	1	3.653e-3	4	NC	1	NC	1
222			min	0	3	-.003	3	-.003	4	-1.617e-4	3	NC	1	5831.101	4
223		17	max	0	1	.003	2	0	1	3.653e-3	4	NC	1	NC	1
224			min	0	3	-.002	3	-.002	4	-1.617e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	3.653e-3	4	NC	1	NC	1
226			min	0	3	-.001	3	0	4	-1.617e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	3.653e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.617e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.007	2	0	3	6.593e-4	1	NC	3	NC	1
230			min	-.003	3	-.006	3	-.005	4	-3.053e-4	3	4899.532	2	NC	1
231		2	max	.002	1	.006	2	0	3	6.255e-4	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.005	4	-2.962e-4	3	5326.664	2	NC	1
233		3	max	.002	1	.006	2	0	3	5.918e-4	1	NC	3	NC	1
234			min	-.003	3	-.006	3	-.005	4	-2.872e-4	3	5831.151	2	NC	1
235		4	max	.002	1	.005	2	0	3	5.581e-4	1	NC	1	NC	1
236			min	-.002	3	-.005	3	-.005	4	-2.781e-4	3	6431.193	2	NC	1
237		5	max	.002	1	.005	2	0	3	6.093e-4	4	NC	1	NC	1
238			min	-.002	3	-.005	3	-.005	4	-2.69e-4	3	7150.914	2	NC	1
239		6	max	.002	1	.004	2	0	3	6.669e-4	4	NC	1	NC	1
240			min	-.002	3	-.005	3	-.005	4	-2.6e-4	3	8022.82	2	NC	1
241		7	max	.002	1	.004	2	0	3	7.245e-4	4	NC	1	NC	1
242			min	-.002	3	-.005	3	-.005	4	-2.509e-4	3	9091.554	2	NC	1
243		8	max	.002	1	.003	2	0	3	7.821e-4	4	NC	1	NC	1
244			min	-.002	3	-.004	3	-.005	4	-2.418e-4	3	NC	1	NC	1
245		9	max	.001	1	.003	2	0	3	8.396e-4	4	NC	1	NC	1
246			min	-.002	3	-.004	3	-.005	4	-2.328e-4	3	NC	1	NC	1
247		10	max	.001	1	.002	2	0	3	8.972e-4	4	NC	1	NC	1
248			min	-.001	3	-.004	3	-.005	4	-2.237e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	9.548e-4	4	NC	1	NC	1
250			min	-.001	3	-.004	3	-.004	4	-2.146e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	1.012e-3	4	NC	1	NC	1
252			min	-.001	3	-.003	3	-.004	4	-2.056e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	1.07e-3	4	NC	1	NC	1
254			min	0	3	-.003	3	-.004	4	-1.965e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	1.128e-3	4	NC	1	NC	1
256			min	0	3	-.002	3	-.003	4	-1.874e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.185e-3	4	NC	1	NC	1
258			min	0	3	-.002	3	-.003	4	-1.784e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.243e-3	4	NC	1	NC	1
260			min	0	3	-.002	3	-.002	4	-1.693e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.3e-3	4	NC	1	NC	1
262			min	0	3	-.001	3	-.001	4	-1.602e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.358e-3	4	NC	1	NC	1
264			min	0	3	0	3	0	4	-1.512e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.416e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.421e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	6.543e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-6.517e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.003	4	4.996e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-7.272e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.007	4	3.449e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-8.028e-4	4	NC	1	6632.735	4
273		4	max	0	3	0	2	.01	4	1.901e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-8.783e-4	4	NC	1	4385.821	4
275		5	max	0	3	0	2	.014	4	3.538e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-9.538e-4	4	NC	1	3268.145	4
277		6	max	0	3	0	2	.018	4	-7.838e-6	12	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-1.029e-3	4	NC	1	2601.46	4
279		7	max	0	3	0	2	.021	5	-1.413e-5	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-1.105e-3	4	NC	1	2156.494	5
281		8	max	0	3	0	2	.025	5	-1.586e-5	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-1.18e-3	4	NC	1	1839.351	5
283		9	max	0	3	.001	2	.029	5	-1.76e-5	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	1	-1.256e-3	4	NC	1	1603.151	5
285		10	max	0	3	.002	2	.032	5	-1.933e-5	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	1	-1.332e-3	4	NC	1	1420.596	5
287		11	max	0	3	.002	2	.036	5	-2.107e-5	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	1	-1.407e-3	4	NC	1	1275.316	5
289		12	max	0	3	.003	2	.04	5	-2.28e-5	10	NC	1	NC	1
290			min	0	2	-.007	3	-.004	1	-1.483e-3	4	NC	1	1156.905	5
291		13	max	0	3	.003	2	.044	5	-2.454e-5	10	NC	1	NC	1
292			min	0	2	-.007	3	-.005	1	-1.558e-3	4	NC	1	1058.429	5
293		14	max	0	3	.004	2	.047	5	-2.627e-5	10	NC	1	NC	2
294			min	0	2	-.007	3	-.005	1	-1.634e-3	4	NC	1	975.092	5
295		15	max	0	3	.005	2	.051	5	-2.801e-5	10	NC	1	NC	2
296			min	0	2	-.007	3	-.006	1	-1.709e-3	4	9746.163	2	903.47	5
297		16	max	0	3	.006	2	.055	5	-2.974e-5	10	NC	1	NC	2
298			min	0	2	-.007	3	-.006	1	-1.785e-3	4	8216.876	2	841.055	5
299		17	max	0	3	.007	2	.059	5	-3.147e-5	10	NC	3	NC	2
300			min	0	2	-.007	3	-.007	1	-1.86e-3	4	7041.55	2	785.972	5
301		18	max	0	3	.008	2	.062	5	-3.321e-5	10	NC	3	NC	2
302			min	0	2	-.007	3	-.008	1	-1.936e-3	4	6127.363	2	736.79	5
303		19	max	0	3	.009	2	.067	5	-3.494e-5	10	NC	3	NC	2
304			min	0	2	-.007	3	-.008	1	-2.011e-3	4	5409.568	2	692.403	5
305	M12	1	max	.002	1	.008	2	.007	1	4.75e-3	4	NC	1	NC	3
306			min	0	3	-.006	3	-.073	5	3.531e-5	10	NC	1	263.924	5
307		2	max	.002	1	.007	2	.006	1	4.75e-3	4	NC	1	NC	3
308			min	0	3	-.006	3	-.067	5	3.531e-5	10	NC	1	287.7	5
309		3	max	.002	1	.007	2	.006	1	4.75e-3	4	NC	1	NC	2
310			min	0	3	-.006	3	-.061	5	3.531e-5	10	NC	1	315.998	5
311		4	max	.002	1	.007	2	.005	1	4.75e-3	4	NC	1	NC	2
312			min	0	3	-.005	3	-.055	5	3.531e-5	10	NC	1	350.009	5
313		5	max	.002	1	.006	2	.005	1	4.75e-3	4	NC	1	NC	2
314			min	0	3	-.005	3	-.049	5	3.531e-5	10	NC	1	391.355	5
315		6	max	.001	1	.006	2	.004	1	4.75e-3	4	NC	1	NC	2
316			min	0	3	-.005	3	-.044	5	3.531e-5	10	NC	1	442.293	5
317		7	max	.001	1	.005	2	.004	1	4.75e-3	4	NC	1	NC	2
318			min	0	3	-.004	3	-.038	5	3.531e-5	10	NC	1	506.035	5
319		8	max	.001	1	.005	2	.003	1	4.75e-3	4	NC	1	NC	2
320			min	0	3	-.004	3	-.033	5	3.531e-5	10	NC	1	587.286	5
321		9	max	.001	1	.004	2	.003	1	4.75e-3	4	NC	1	NC	2
322			min	0	3	-.004	3	-.028	5	3.531e-5	10	NC	1	693.163	5
323		10	max	.001	1	.004	2	.002	1	4.75e-3	4	NC	1	NC	2
324			min	0	3	-.003	3	-.023	5	3.531e-5	10	NC	1	834.852	5
325		11	max	0	1	.003	2	.002	1	4.75e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.019	5	3.531e-5	10	NC	1	1030.775	5
327		12	max	0	1	.003	2	.001	1	4.75e-3	4	NC	1	NC	1



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Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.002	3	-.015	5	3.531e-5	10	NC	1	1313.067	5
329		max	0	1	.003	2	.001	1	4.75e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.011	5	3.531e-5	10	NC	1	1742.004	5
331		max	0	1	.002	2	0	1	4.75e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.008	5	3.531e-5	10	NC	1	2441.914	5
333		max	0	1	.002	2	0	1	4.75e-3	4	NC	1	NC	1
334		min	0	3	-.001	3	-.005	5	3.531e-5	10	NC	1	3704.88	5
335		max	0	1	.001	2	0	1	4.75e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.003	5	3.531e-5	10	NC	1	6361.215	5
337		max	0	1	0	2	0	1	4.75e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.001	5	3.531e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	4.75e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	3.531e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	4.75e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	3.531e-5	10	NC	1	NC	1
343	M1	max	.006	3	.022	3	.007	5	1.557e-2	1	NC	1	NC	1
344		min	-.007	2	-.022	1	-.004	1	-1.725e-2	3	NC	1	NC	1
345		max	.006	3	.012	3	.01	5	7.518e-3	1	NC	4	NC	1
346		min	-.007	2	-.012	1	-.007	1	-8.527e-3	3	4601.369	1	NC	1
347		max	.006	3	.003	3	.014	5	3.654e-4	5	NC	4	NC	2
348		min	-.007	2	-.003	1	-.009	1	-3.872e-4	1	2373.387	1	7022.117	5
349		max	.006	3	.006	2	.017	5	3.645e-4	5	NC	4	NC	2
350		min	-.007	2	-.004	3	-.01	1	-3.262e-4	1	1676.657	1	4451.248	5
351		max	.006	3	.012	1	.022	5	3.636e-4	5	NC	5	NC	2
352		min	-.007	2	-.011	3	-.01	1	-2.652e-4	1	1341.73	1	3196.216	5
353		max	.006	3	.018	1	.026	5	3.628e-4	5	NC	5	NC	2
354		min	-.007	2	-.016	3	-.009	1	-2.042e-4	1	1152.284	1	2462.409	5
355		max	.006	3	.022	1	.03	5	3.619e-4	5	NC	5	NC	2
356		min	-.007	2	-.019	3	-.008	1	-1.433e-4	1	1037.403	1	1986.384	5
357		max	.006	3	.025	2	.035	5	3.611e-4	5	NC	5	NC	1
358		min	-.007	2	-.022	3	-.007	1	-8.229e-5	1	967.598	1	1655.85	5
359		max	.006	3	.028	2	.04	5	3.602e-4	5	NC	5	NC	1
360		min	-.007	2	-.023	3	-.005	1	-2.131e-5	1	927.941	2	1410.069	4
361		max	.006	3	.028	2	.045	5	3.693e-4	4	NC	5	NC	1
362		min	-.007	2	-.023	3	-.003	1	1.087e-5	10	908.09	2	1214.99	4
363		max	.006	3	.028	2	.05	4	3.813e-4	4	NC	5	NC	1
364		min	-.007	2	-.023	3	0	1	1.458e-5	10	911.254	2	1066.704	4
365		max	.006	3	.026	2	.055	4	3.934e-4	4	NC	5	NC	1
366		min	-.007	2	-.021	3	0	10	1.829e-5	10	938.619	2	951.508	4
367		max	.006	3	.023	2	.061	4	4.055e-4	4	NC	5	NC	2
368		min	-.007	2	-.018	3	0	10	2.2e-5	10	995.588	2	860.515	4
369		max	.006	3	.018	2	.066	4	4.176e-4	4	NC	5	NC	2
370		min	-.007	2	-.014	3	0	10	2.571e-5	10	1094.466	2	787.756	4
371		max	.006	3	.012	2	.07	4	4.297e-4	4	NC	4	NC	2
372		min	-.007	2	-.009	3	0	10	2.942e-5	10	1262.086	2	729.102	4
373		max	.006	3	.005	2	.075	4	6.907e-4	4	NC	4	NC	2
374		min	-.007	2	-.004	3	0	10	3.016e-5	12	1563.514	2	681.626	4
375		max	.006	3	.002	3	.079	4	6.876e-3	4	NC	4	NC	2
376		min	-.007	2	-.004	2	0	10	1.366e-5	10	2202.334	2	643.255	4
377		max	.006	3	.009	3	.083	4	9.074e-3	2	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-4.026e-3	3	4258.612	2	612.379	4
379		max	.006	3	.017	3	.086	4	1.829e-2	2	NC	1	NC	1
380		min	-.007	2	-.026	2	-.002	1	-8.161e-3	3	NC	1	588.53	4
381	M5	max	.019	3	.072	3	.007	5	7.212e-6	4	NC	1	NC	1
382		min	-.024	2	-.074	1	-.004	1	4.51e-8	2	NC	1	NC	1
383		max	.019	3	.04	3	.01	5	1.779e-4	5	NC	5	NC	1
384		min	-.024	2	-.04	1	-.004	1	-7.736e-5	1	1362.033	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.019	3	.01	3	.013	5	3.459e-4	5	NC	5	NC	1
386		min	-.024	2	-.009	1	-.004	1	-1.533e-4	1	701.845	1	NC	1
387	4	max	.019	3	.019	1	.018	5	3.598e-4	5	NC	5	NC	1
388		min	-.024	2	-.014	3	-.004	1	-1.457e-4	1	494.912	1	NC	1
389	5	max	.019	3	.042	1	.022	5	3.738e-4	5	NC	5	NC	1
390		min	-.024	2	-.035	3	-.003	1	-1.382e-4	1	395.315	1	NC	1
391	6	max	.019	3	.061	1	.027	5	3.877e-4	5	NC	5	NC	1
392		min	-.024	2	-.051	3	-.003	1	-1.307e-4	1	338.881	1	NC	1
393	7	max	.019	3	.076	1	.032	5	4.016e-4	5	NC	15	NC	1
394		min	-.024	2	-.063	3	-.003	1	-1.231e-4	1	304.558	1	NC	1
395	8	max	.019	3	.087	1	.037	5	4.156e-4	5	NC	15	NC	1
396		min	-.024	2	-.071	3	-.003	1	-1.156e-4	1	283.586	1	NC	1
397	9	max	.018	3	.094	1	.042	5	4.295e-4	5	NC	15	NC	1
398		min	-.024	2	-.075	3	-.003	1	-1.081e-4	1	271.864	1	NC	1
399	10	max	.018	3	.096	1	.047	5	4.435e-4	5	NC	15	NC	1
400		min	-.024	2	-.076	3	-.003	1	-1.006e-4	1	267.487	1	NC	1
401	11	max	.018	3	.094	1	.053	5	4.574e-4	5	NC	15	NC	1
402		min	-.024	2	-.073	3	-.003	1	-9.302e-5	1	269.847	1	NC	1
403	12	max	.018	3	.087	1	.058	4	4.713e-4	5	NC	15	NC	1
404		min	-.024	2	-.067	3	-.002	1	-8.549e-5	1	279.399	1	NC	1
405	13	max	.018	3	.077	2	.063	4	4.853e-4	5	NC	15	NC	1
406		min	-.024	2	-.058	3	-.002	1	-7.796e-5	1	297.804	2	NC	1
407	14	max	.018	3	.062	2	.068	4	4.992e-4	5	NC	5	NC	1
408		min	-.024	2	-.046	3	-.002	1	-7.043e-5	1	327.348	2	NC	1
409	15	max	.018	3	.042	2	.072	4	5.132e-4	5	NC	5	NC	1
410		min	-.024	2	-.031	3	-.002	1	-6.29e-5	1	377.512	2	NC	1
411	16	max	.018	3	.017	2	.076	4	7.721e-4	5	NC	5	NC	1
412		min	-.024	2	-.013	3	-.002	1	-5.904e-5	1	467.863	2	NC	1
413	17	max	.018	3	.008	3	.08	4	6.898e-3	4	NC	5	NC	1
414		min	-.024	2	-.014	2	-.002	1	-1.426e-4	1	659.994	2	NC	1
415	18	max	.018	3	.03	3	.083	4	3.54e-3	4	NC	5	NC	1
416		min	-.024	2	-.049	2	-.002	1	-7.276e-5	1	1277.121	2	NC	1
417	19	max	.018	3	.054	3	.086	4	2.624e-6	5	NC	1	NC	1
418		min	-.024	2	-.087	2	-.002	1	-2.351e-7	3	NC	1	NC	1
419	M9	1	max	.006	.022	3	.006	5	1.726e-2	3	NC	1	NC	1
420		min	-.007	2	-.022	1	-.004	1	-1.557e-2	1	NC	1	NC	1
421	2	max	.006	3	.012	3	.005	5	8.549e-3	3	NC	4	NC	1
422		min	-.007	2	-.012	1	0	1	-7.678e-3	1	4602.927	1	NC	1
423	3	max	.006	3	.003	3	.005	4	6.934e-5	1	NC	4	NC	1
424		min	-.007	2	-.003	1	0	3	-1.969e-5	5	2374.215	1	NC	1
425	4	max	.006	3	.005	2	.007	4	2.511e-5	2	NC	4	NC	1
426		min	-.007	2	-.005	3	0	3	-3.032e-5	5	1677.245	1	NC	1
427	5	max	.006	3	.012	1	.009	4	7.829e-6	10	NC	5	NC	1
428		min	-.007	2	-.011	3	-.002	3	-4.759e-5	4	1342.183	1	9070.563	14
429	6	max	.006	3	.018	1	.012	4	4.111e-6	10	NC	5	NC	1
430		min	-.007	2	-.016	3	-.002	3	-7.283e-5	1	1152.652	1	6444.221	4
431	7	max	.006	3	.022	1	.015	4	3.931e-7	10	NC	5	NC	1
432		min	-.007	2	-.019	3	-.002	3	-1.202e-4	1	1037.713	1	4342.752	4
433	8	max	.006	3	.025	2	.019	4	-3.325e-6	10	NC	5	NC	1
434		min	-.007	2	-.022	3	-.003	3	-1.676e-4	1	967.866	1	3148.296	4
435	9	max	.006	3	.028	2	.024	4	-7.042e-6	10	NC	5	NC	1
436		min	-.007	2	-.023	3	-.003	3	-2.15e-4	1	928.439	2	2404.007	4
437	10	max	.006	3	.028	2	.029	5	-1.076e-5	10	NC	5	NC	1
438		min	-.007	2	-.024	3	-.003	3	-2.624e-4	1	908.567	2	1908.358	4
439	11	max	.006	3	.028	2	.035	5	-1.448e-5	10	NC	5	NC	1
440		min	-.007	2	-.023	3	-.004	1	-3.098e-4	1	911.723	2	1561.313	4
441	12	max	.006	3	.026	2	.041	5	-1.82e-5	10	NC	5	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.132	3	.042	1	6.502e-3	2	NC	5	NC	3
500		min	-.086	4	-.283	2	0	10	-4.172e-3	3	559.404	2	3106.206	1
501	4	max	.002	1	.166	3	.064	1	7.614e-3	2	NC	5	NC	3
502		min	-.086	4	-.357	2	0	10	-4.861e-3	3	434.67	2	2121.355	1
503	5	max	.002	1	.177	3	.073	1	8.726e-3	2	NC	5	NC	3
504		min	-.086	4	-.381	2	0	10	-5.551e-3	3	405.817	2	1859.936	1
505	6	max	.002	1	.168	3	.068	1	9.838e-3	2	NC	5	NC	5
506		min	-.086	4	-.355	2	-.002	10	-6.241e-3	3	437.777	2	2000.286	1
507	7	max	.002	1	.14	3	.048	1	1.095e-2	2	NC	5	NC	2
508		min	-.086	4	-.289	2	-.005	10	-6.931e-3	3	546.767	2	2731.141	1
509	8	max	.002	1	.104	3	.021	3	1.206e-2	2	NC	5	NC	2
510		min	-.086	4	-.203	2	-.009	10	-7.621e-3	3	814.157	2	5782.606	1
511	9	max	.002	1	.07	3	.02	3	1.317e-2	2	NC	5	NC	1
512		min	-.086	4	-.123	2	-.019	2	-8.31e-3	3	1482.717	2	NC	1
513	10	max	.002	1	.054	3	.018	3	1.429e-2	2	NC	4	NC	3
514		min	-.086	4	-.087	2	-.024	2	-9.e-3	3	2371.303	2	8373.057	2
515	11	max	.002	1	.07	3	.018	3	1.317e-2	2	NC	5	NC	1
516		min	-.086	4	-.123	2	-.019	2	-8.31e-3	3	1482.717	2	NC	1
517	12	max	.002	1	.104	3	.021	1	1.206e-2	2	NC	5	NC	2
518		min	-.086	4	-.203	2	-.009	10	-7.619e-3	3	814.157	2	5730.62	1
519	13	max	.002	1	.14	3	.048	1	1.095e-2	2	NC	5	NC	2
520		min	-.086	4	-.289	2	-.005	10	-6.929e-3	3	546.767	2	2725.857	1
521	14	max	.002	1	.168	3	.068	1	9.839e-3	2	NC	5	NC	3
522		min	-.086	4	-.355	2	-.002	10	-6.238e-3	3	437.777	2	2002.946	1
523	15	max	.002	1	.177	3	.073	1	8.727e-3	2	NC	5	NC	3
524		min	-.086	4	-.381	2	-.003	5	-5.548e-3	3	405.817	2	1867.286	1
525	16	max	.002	1	.166	3	.063	1	7.615e-3	2	NC	5	NC	3
526		min	-.086	4	-.357	2	-.007	5	-4.858e-3	3	434.67	2	2135.908	1
527	17	max	.002	1	.132	3	.041	1	6.504e-3	2	NC	5	NC	3
528		min	-.086	4	-.283	2	-.009	5	-4.167e-3	3	559.404	2	3140.492	1
529	18	max	.002	1	.08	3	.015	1	5.392e-3	2	NC	5	NC	2
530		min	-.086	4	-.167	2	-.007	5	-3.477e-3	3	1019.004	2	7344.496	1
531	19	max	.002	1	.017	3	.006	3	4.28e-3	2	NC	1	NC	1
532		min	-.086	4	-.026	2	-.007	2	-2.786e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.264e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.231e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.007	4	8.244e-4	3	NC	1	NC	1
536		min	0	5	-.007	1	0	3	-5.935e-4	2	NC	1	NC	1
537	3	max	0	3	0	15	.015	4	1.322e-3	3	NC	5	NC	1
538		min	-.001	5	-.013	1	-.003	3	-1.159e-3	1	6112.169	1	5418.323	4
539	4	max	0	3	0	15	.022	4	1.82e-3	3	NC	5	NC	9
540		min	-.002	5	-.019	1	-.007	3	-1.729e-3	1	4193.305	1	3557.511	4
541	5	max	0	3	0	15	.029	4	2.318e-3	3	NC	5	NC	9
542		min	-.003	5	-.025	1	-.011	3	-2.299e-3	1	3272.078	1	2707.153	4
543	6	max	0	3	0	15	.035	4	2.816e-3	3	NC	5	8129.673	9
544		min	-.003	5	-.029	1	-.016	3	-2.869e-3	1	2753.8	1	2256.929	4
545	7	max	0	3	0	15	.039	4	3.314e-3	3	NC	5	6379.513	9
546		min	-.004	5	-.033	1	-.02	3	-3.44e-3	1	2442.124	1	2010.679	4
547	8	max	0	3	0	15	.042	4	3.812e-3	3	NC	5	5275.386	9
548		min	-.005	5	-.036	1	-.025	3	-4.01e-3	1	2255.071	1	1890.634	4
549	9	max	0	3	0	15	.042	4	4.31e-3	3	NC	5	4551.217	9
550		min	-.005	5	-.038	1	-.029	3	-4.58e-3	1	2154.388	1	1865.306	4
551	10	max	0	3	0	15	.041	4	4.808e-3	3	NC	5	4072.838	9
552		min	-.006	5	-.039	1	-.033	3	-5.151e-3	1	2122.537	1	1748.685	1
553	11	max	0	3	0	15	.038	1	5.306e-3	3	NC	5	3795.213	15
554		min	-.007	5	-.038	1	-.035	3	-5.721e-3	1	2154.388	1	1615.629	1
555	12	max	0	3	0	15	.039	1	5.804e-3	3	NC	5	4531.923	15





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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.