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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

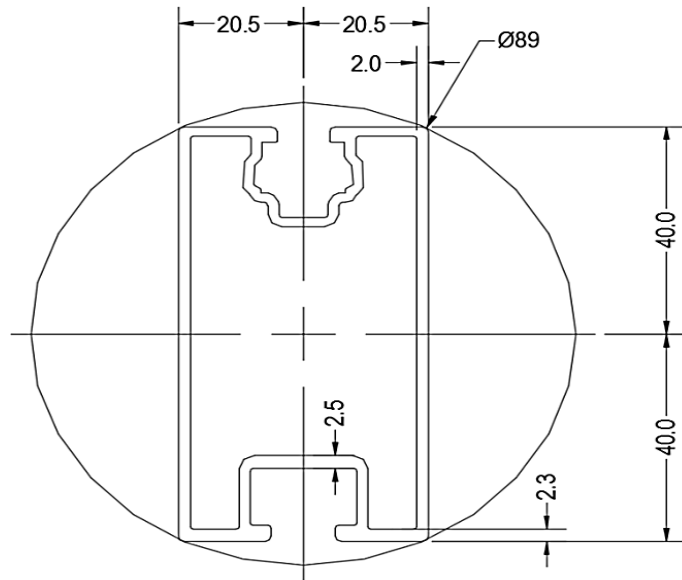
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

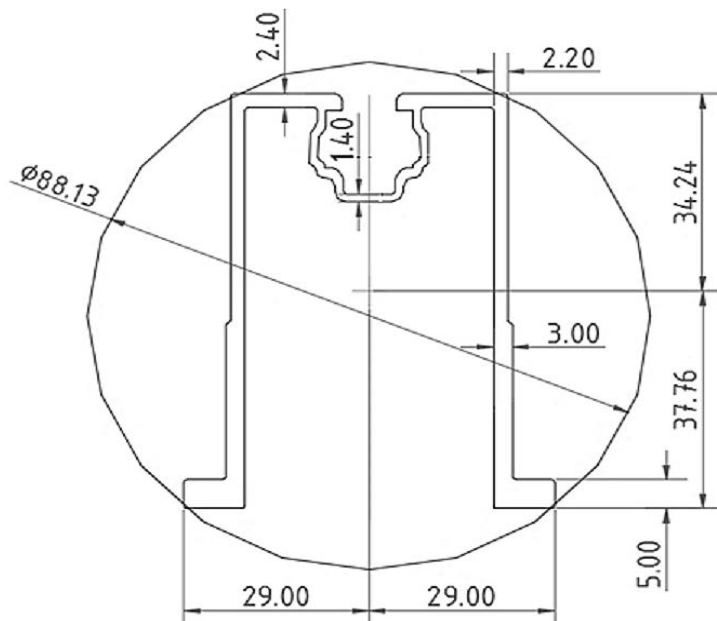
Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	28.61 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	1.344 k-ft
$M_z$ =	0.193 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>99%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.93 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.655 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.201 k
$M_{y \text{ allowable}}$ =	1.469 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>46%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	1.776 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>15%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.151 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>4%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

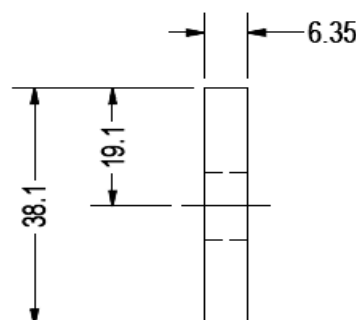
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.433 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>18%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.007 k-ft
$P_n$ =	0.032 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>16%</b>



A cross brace kit is required every 12 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

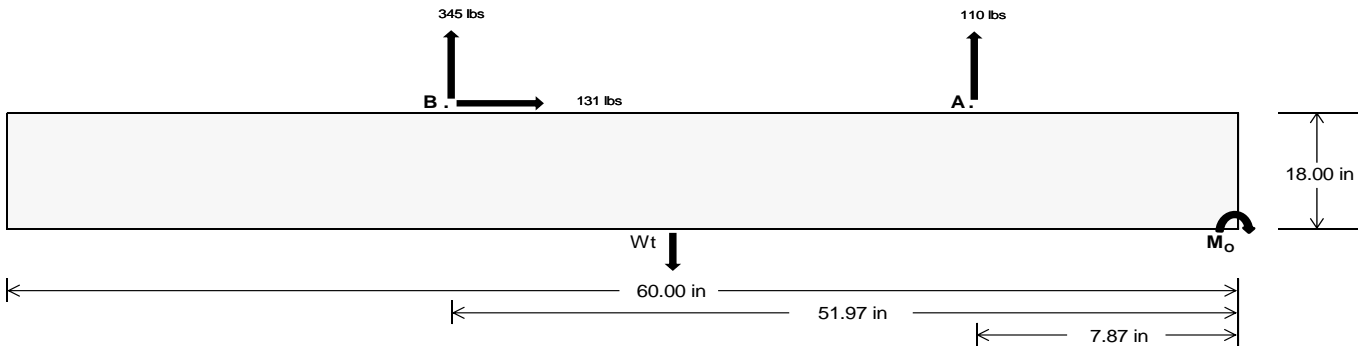
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>486.43</b>	<b>1503.16</b>	k
Compressive Load =	<b>2308.74</b>	<b>1694.29</b>	k
Lateral Load =	<b>4.36</b>	<b>566.11</b>	k
Moment (Weak Axis) =	<b>0.01</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21156.3$  in-lbs  
Resisting Force Required = 705.21 lbs  
S.F. = 1.67  
Weight Required = 1175.35 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 130.53 lbs  
Friction = 0.4  
Weight Required = 326.33 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 130.53 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F <sub>A</sub>	885 lbs	885 lbs	885 lbs	885 lbs	611 lbs	611 lbs	611 lbs	611 lbs	1062 lbs	1062 lbs	1062 lbs	1062 lbs	-220 lbs	-220 lbs	-220 lbs	-220 lbs
F <sub>B</sub>	652 lbs	652 lbs	652 lbs	652 lbs	447 lbs	447 lbs	447 lbs	447 lbs	778 lbs	778 lbs	778 lbs	778 lbs	-690 lbs	-690 lbs	-690 lbs	-690 lbs
F <sub>V</sub>	62 lbs	62 lbs	62 lbs	62 lbs	234 lbs	234 lbs	234 lbs	234 lbs	218 lbs	218 lbs	218 lbs	218 lbs	-261 lbs	-261 lbs	-261 lbs	-261 lbs
P <sub>total</sub>	3440 lbs	3531 lbs	3622 lbs	3712 lbs	2961 lbs	3052 lbs	3142 lbs	3233 lbs	3743 lbs	3834 lbs	3925 lbs	4015 lbs	231 lbs	286 lbs	340 lbs	395 lbs
M	532 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	861 lbs-ft	861 lbs-ft	861 lbs-ft	861 lbs-ft	467 lbs-ft	467 lbs-ft	467 lbs-ft	467 lbs-ft
e	0.15 ft	0.15 ft	0.15 ft	0.14 ft	0.22 ft	0.22 ft	0.21 ft	0.20 ft	0.23 ft	0.22 ft	0.22 ft	0.21 ft	2.02 ft	1.63 ft	1.37 ft	1.18 ft
L/6	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f <sub>min</sub>	320.2 psf	315.5 psf	311.2 psf	307.3 psf	248.0 psf	246.6 psf	245.4 psf	244.2 psf	309.8 psf	305.6 psf	301.8 psf	298.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f <sub>max</sub>	466.2 psf	454.9 psf	444.6 psf	435.1 psf	428.8 psf	419.2 psf	410.4 psf	402.4 psf	545.8 psf	530.9 psf	517.3 psf	504.8 psf	182.4 psf	119.9 psf	104.9 psf	99.9 psf

Maximum Bearing Pressure = 546 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

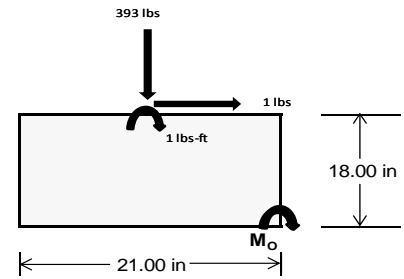
## Overturning Check

$M_o = 341.8 \text{ ft-lbs}$   
 Resisting Force Required = 390.64 lbs  
 S.F. = 1.67  
 Weight Required = 651.06 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	93 lbs	263 lbs	88 lbs	398 lbs	1244 lbs	393 lbs	27 lbs	77 lbs	26 lbs
$F_v$	4 lbs	3 lbs	0 lbs	18 lbs	18 lbs	1 lbs	1 lbs	1 lbs	0 lbs
$P_{total}$	2449 lbs	2619 lbs	2444 lbs	2641 lbs	3487 lbs	2636 lbs	716 lbs	766 lbs	715 lbs
$M$	5 lbs-ft	5 lbs-ft	0 lbs-ft	32 lbs-ft	26 lbs-ft	2 lbs-ft	2 lbs-ft	1 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.73 ft	1.73 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
$f_{min}$	277.8 sqft	297.3 sqft	279.2 sqft	289.5 sqft	388.2 sqft	300.4 sqft	81.2 sqft	86.9 sqft	81.6 sqft
$f_{max}$	282.0 psf	301.3 psf	279.4 psf	314.2 psf	408.8 psf	302.2 psf	82.5 psf	88.1 psf	81.7 psf



Maximum Bearing Pressure = 409 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

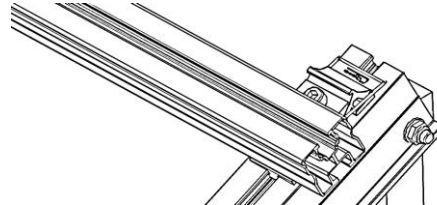
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.357 k
Allowable Uplift =	1.214 k
Utilization =	<u>29%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.058 k
Allowable Uplift =	1.116 k
Utilization =	<u>95%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.776 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>31%</u>

#### Diagonal Strut

Maximum Axial Load =	0.151 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.433 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

#### Bracing

Maximum Axial Load =	0.032 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.053 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.45 \\
 &20.4426 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.45 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.469 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



# A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

## 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

## 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

## 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

## 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

## 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

## 3.4.16.1

N/A for Weak Direction

## 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

## 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.248	-63.248	0	0
2	M16	Y	-63.248	-63.248	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-52.98	-52.98	0	0
2	M16	y	-84.769	-84.769	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	108.08	108.08	0	0
2	M16	y	52.98	52.98	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								
8																				
9	ASD 1.0D + 1.0S	Yes	Y		1	1	3	1												
10	ASD 1.0D + 0.6W	Yes	Y		1	1			4	.6										
11	ASD 1.0D + 0.75L + 0.45W + 0....	Yes	Y		1	1	3	.75	4	.45										
12	ASD 0.6D + 0.6W	Yes	Y		2	.6					5	.6								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y		1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y		1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y		1	.362					6	.875								



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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	96.702	2	362.375	1	.029	2	0	1	0	1	0	1
2		min	-136.404	3	-351.109	3	-.099	3	0	3	0	1	0	1
3	N7	max	0	15	596.231	1	-.054	15	0	15	0	1	0	1
4		min	-.193	1	-106.31	3	-1.497	1	-.003	1	0	1	0	1
5	N15	max	0	15	1775.955	1	.511	1	.001	1	0	1	0	1
6		min	-2.076	1	-374.174	3	-.209	3	0	3	0	1	0	1
7	N16	max	414.162	2	1303.301	1	-.228	10	0	1	0	1	0	1
8		min	-435.466	3	-1156.28	3	-29.591	1	0	3	0	1	0	1
9	N23	max	0	15	596.137	1	3.353	1	.006	1	0	1	0	1
10		min	-.193	1	-105.903	3	.113	15	0	15	0	1	0	1
11	N24	max	97.116	2	367.98	1	27.319	1	.002	1	0	1	0	1
12		min	-136.448	3	-348.02	3	.046	10	0	3	0	1	0	1
13	Totals:	max	606.085	2	5001.979	1	0	1						
14		min	-708.652	3	-2441.795	3	0	10						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	435.476	1	.659	4	.912	1	0	15	0	3	0	1
2			min	-352.758	3	.157	15	-.047	3	-.001	1	0	2	0	1
3		2	max	435.573	1	.622	4	.912	1	0	15	0	1	0	15
4			min	-352.686	3	.148	15	-.047	3	-.001	1	0	10	0	4
5		3	max	435.669	1	.584	4	.912	1	0	15	0	1	0	15
6			min	-352.613	3	.139	15	-.047	3	-.001	1	0	15	0	4
7		4	max	435.765	1	.546	4	.912	1	0	15	0	1	0	15
8			min	-352.541	3	.13	15	-.047	3	-.001	1	0	12	0	4
9		5	max	435.862	1	.508	4	.912	1	0	15	0	1	0	15
10			min	-352.469	3	.121	15	-.047	3	-.001	1	0	3	0	4
11		6	max	435.958	1	.47	4	.912	1	0	15	0	1	0	15
12			min	-352.396	3	.112	15	-.047	3	-.001	1	0	3	0	4
13		7	max	436.055	1	.433	4	.912	1	0	15	0	1	0	15
14			min	-352.324	3	.103	15	-.047	3	-.001	1	0	3	0	4
15		8	max	436.151	1	.395	4	.912	1	0	15	0	1	0	15
16			min	-352.252	3	.095	15	-.047	3	-.001	1	0	3	0	4
17		9	max	436.247	1	.357	4	.912	1	0	15	.001	1	0	15
18			min	-352.18	3	.086	15	-.047	3	-.001	1	0	3	0	4
19		10	max	436.344	1	.319	4	.912	1	0	15	.001	1	0	15
20			min	-352.107	3	.077	15	-.047	3	-.001	1	0	3	0	4
21		11	max	436.44	1	.281	4	.912	1	0	15	.001	1	0	15
22			min	-352.035	3	.068	15	-.047	3	-.001	1	0	3	0	4
23		12	max	436.536	1	.243	4	.912	1	0	15	.001	1	0	15
24			min	-351.963	3	.059	15	-.047	3	-.001	1	0	3	0	4
25		13	max	436.633	1	.206	4	.912	1	0	15	.002	1	0	15
26			min	-351.891	3	.05	15	-.047	3	-.001	1	0	3	0	4
27		14	max	436.729	1	.168	4	.912	1	0	15	.002	1	0	15
28			min	-351.818	3	.041	15	-.047	3	-.001	1	0	3	0	4
29		15	max	436.825	1	.13	4	.912	1	0	15	.002	1	0	15
30			min	-351.746	3	.032	15	-.047	3	-.001	1	0	3	0	4
31		16	max	436.922	1	.092	4	.912	1	0	15	.002	1	0	15
32			min	-351.674	3	.023	15	-.047	3	-.001	1	0	3	0	4
33		17	max	437.018	1	.054	4	.912	1	0	15	.002	1	0	15
34			min	-351.601	3	-.003	1	-.047	3	-.001	1	0	3	0	4
35		18	max	437.115	1	.028	10	.912	1	0	15	.002	1	0	15
36			min	-351.529	3	-.032	1	-.047	3	-.001	1	0	3	0	4
37		19	max	437.211	1	.004	10	.912	1	0	15	.002	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-351.457	3	-.062	1	-.047	3	-.001	1	0	3	0	4
39	M3	1	max	32.303	10	1.811	4	-.024	15	0	.002	1	0	4
40		min	-130.842	1	.427	15	-.79	1	0	1	0	15	0	15
41		2	max	32.248	10	1.633	4	-.024	15	0	.002	1	0	4
42		min	-130.909	1	.385	15	-.79	1	0	1	0	15	0	15
43		3	max	32.192	10	1.455	4	-.024	15	0	.002	1	0	10
44		min	-130.976	1	.343	15	-.79	1	0	1	0	15	0	1
45		4	max	32.136	10	1.277	4	-.024	15	0	.002	1	0	15
46		min	-131.043	1	.301	15	-.79	1	0	1	0	15	0	1
47		5	max	32.08	10	1.099	4	-.024	15	0	.002	1	0	15
48		min	-131.11	1	.259	15	-.79	1	0	1	0	15	0	4
49		6	max	32.024	10	.921	4	-.024	15	0	.001	1	0	15
50		min	-131.177	1	.218	15	-.79	1	0	1	0	15	0	4
51		7	max	31.968	10	.743	4	-.024	15	0	.001	1	0	15
52		min	-131.244	1	.176	15	-.79	1	0	1	0	15	0	4
53		8	max	31.912	10	.565	4	-.024	15	0	.001	1	0	15
54		min	-131.311	1	.134	15	-.79	1	0	1	0	15	0	4
55		9	max	31.856	10	.387	4	-.024	15	0	0	1	0	15
56		min	-131.378	1	.092	15	-.79	1	0	1	0	15	-.001	4
57		10	max	31.8	10	.209	4	-.024	15	0	0	1	0	15
58		min	-131.445	1	.05	15	-.79	1	0	1	0	15	-.001	4
59		11	max	31.744	10	.031	10	-.024	15	0	0	1	0	15
60		min	-131.513	1	-.006	1	-.79	1	0	1	0	12	-.001	4
61		12	max	31.688	10	-.033	15	-.024	15	0	0	1	0	15
62		min	-131.58	1	-.147	4	-.79	1	0	1	0	12	-.001	4
63		13	max	31.633	10	-.075	15	-.024	15	0	0	1	0	15
64		min	-131.647	1	-.325	4	-.79	1	0	1	0	12	-.001	4
65		14	max	31.577	10	-.117	15	-.024	15	0	0	1	0	15
66		min	-131.714	1	-.503	4	-.79	1	0	1	0	3	-.001	4
67		15	max	31.521	10	-.159	15	-.024	15	0	0	15	0	15
68		min	-131.781	1	-.681	4	-.79	1	0	1	0	1	0	4
69		16	max	31.465	10	-.201	15	-.024	15	0	0	15	0	15
70		min	-131.848	1	-.859	4	-.79	1	0	1	0	1	0	4
71		17	max	31.409	10	-.243	15	-.024	15	0	0	15	0	15
72		min	-131.915	1	-1.037	4	-.79	1	0	1	0	1	0	4
73		18	max	31.353	10	-.285	15	-.024	15	0	0	15	0	15
74		min	-131.982	1	-1.216	4	-.79	1	0	1	0	1	0	4
75		19	max	31.297	10	-.326	15	-.024	15	0	0	15	0	1
76		min	-132.049	1	-1.394	4	-.79	1	0	1	0	1	0	1
77	M4	1	max	595.067	1	0	1	-.054	15	0	0	3	0	1
78		min	-107.184	3	0	1	-1.662	1	0	1	0	1	0	1
79		2	max	595.131	1	0	1	-.054	15	0	0	12	0	1
80		min	-107.135	3	0	1	-1.662	1	0	1	0	1	0	1
81		3	max	595.196	1	0	1	-.054	15	0	0	15	0	1
82		min	-107.086	3	0	1	-1.662	1	0	1	0	1	0	1
83		4	max	595.261	1	0	1	-.054	15	0	0	15	0	1
84		min	-107.038	3	0	1	-1.662	1	0	1	0	1	0	1
85		5	max	595.326	1	0	1	-.054	15	0	0	15	0	1
86		min	-106.989	3	0	1	-1.662	1	0	1	0	1	0	1
87		6	max	595.39	1	0	1	-.054	15	0	0	15	0	1
88		min	-106.941	3	0	1	-1.662	1	0	1	0	1	0	1
89		7	max	595.455	1	0	1	-.054	15	0	0	15	0	1
90		min	-106.892	3	0	1	-1.662	1	0	1	0	1	0	1
91		8	max	595.52	1	0	1	-.054	15	0	0	15	0	1
92		min	-106.844	3	0	1	-1.662	1	0	1	-.001	1	0	1
93		9	max	595.584	1	0	1	-.054	15	0	0	15	0	1
94		min	-106.795	3	0	1	-1.662	1	0	1	-.001	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	595.649	1	0	1	-.054	15	0	1	0	15	0	1
96			min	-106.747	3	0	1	-1.662	1	0	1	-.001	1	0	1
97		11	max	595.714	1	0	1	-.054	15	0	1	0	15	0	1
98			min	-106.698	3	0	1	-1.662	1	0	1	-.002	1	0	1
99		12	max	595.779	1	0	1	-.054	15	0	1	0	15	0	1
100			min	-106.65	3	0	1	-1.662	1	0	1	-.002	1	0	1
101		13	max	595.843	1	0	1	-.054	15	0	1	0	15	0	1
102			min	-106.601	3	0	1	-1.662	1	0	1	-.002	1	0	1
103		14	max	595.908	1	0	1	-.054	15	0	1	0	15	0	1
104			min	-106.553	3	0	1	-1.662	1	0	1	-.002	1	0	1
105		15	max	595.973	1	0	1	-.054	15	0	1	0	15	0	1
106			min	-106.504	3	0	1	-1.662	1	0	1	-.002	1	0	1
107		16	max	596.037	1	0	1	-.054	15	0	1	0	15	0	1
108			min	-106.456	3	0	1	-1.662	1	0	1	-.002	1	0	1
109		17	max	596.102	1	0	1	-.054	15	0	1	0	15	0	1
110			min	-106.407	3	0	1	-1.662	1	0	1	-.002	1	0	1
111		18	max	596.167	1	0	1	-.054	15	0	1	0	15	0	1
112			min	-106.359	3	0	1	-1.662	1	0	1	-.003	1	0	1
113		19	max	596.231	1	0	1	-.054	15	0	1	0	15	0	1
114			min	-106.31	3	0	1	-1.662	1	0	1	-.003	1	0	1
115	M6	1	max	1431.416	1	.642	4	.347	1	0	1	0	3	0	1
116			min	-1159.118	3	.154	15	-.11	3	0	15	0	1	0	1
117		2	max	1431.512	1	.604	4	.347	1	0	1	0	3	0	15
118			min	-1159.046	3	.146	15	-.11	3	0	15	0	2	0	4
119		3	max	1431.608	1	.566	4	.347	1	0	1	0	1	0	15
120			min	-1158.973	3	.137	15	-.11	3	0	15	0	12	0	4
121		4	max	1431.705	1	.528	4	.347	1	0	1	0	1	0	15
122			min	-1158.901	3	.128	15	-.11	3	0	15	0	3	0	4
123		5	max	1431.801	1	.49	4	.347	1	0	1	0	1	0	15
124			min	-1158.829	3	.119	15	-.11	3	0	15	0	3	0	4
125		6	max	1431.897	1	.452	4	.347	1	0	1	0	1	0	15
126			min	-1158.756	3	.11	15	-.11	3	0	15	0	3	0	4
127		7	max	1431.994	1	.415	4	.347	1	0	1	0	1	0	15
128			min	-1158.684	3	.101	15	-.11	3	0	15	0	3	0	4
129		8	max	1432.09	1	.377	4	.347	1	0	1	0	1	0	15
130			min	-1158.612	3	.092	15	-.11	3	0	15	0	3	0	4
131		9	max	1432.186	1	.339	4	.347	1	0	1	0	1	0	15
132			min	-1158.54	3	.083	15	-.11	3	0	15	0	3	0	4
133		10	max	1432.283	1	.301	4	.347	1	0	1	0	1	0	15
134			min	-1158.467	3	.074	15	-.11	3	0	15	0	3	0	4
135		11	max	1432.379	1	.263	4	.347	1	0	1	0	1	0	15
136			min	-1158.395	3	.066	15	-.11	3	0	15	0	3	0	4
137		12	max	1432.476	1	.225	4	.347	1	0	1	0	1	0	15
138			min	-1158.323	3	.057	15	-.11	3	0	15	0	3	0	4
139		13	max	1432.572	1	.188	4	.347	1	0	1	0	1	0	15
140			min	-1158.251	3	.048	15	-.11	3	0	15	0	3	0	4
141		14	max	1432.668	1	.15	2	.347	1	0	1	0	1	0	15
142			min	-1158.178	3	.03	9	-.11	3	0	15	0	3	0	4
143		15	max	1432.765	1	.121	2	.347	1	0	1	0	1	0	15
144			min	-1158.106	3	.006	9	-.11	3	0	15	0	3	0	4
145		16	max	1432.861	1	.094	10	.347	1	0	1	0	1	0	15
146			min	-1158.034	3	-.023	1	-.11	3	0	15	0	3	0	4
147		17	max	1432.957	1	.069	10	.347	1	0	1	0	1	0	15
148			min	-1157.961	3	-.053	1	-.11	3	0	15	0	3	0	4
149		18	max	1433.054	1	.045	10	.347	1	0	1	0	1	0	15
150			min	-1157.889	3	-.082	1	-.11	3	0	15	0	3	0	4
151		19	max	1433.15	1	.02	10	.347	1	0	1	0	1	0	15





Company : Schletter, Inc.  
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Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1157.817	3	-.112	1	-.11	3	0	15	0	3	0	4
153		max	151.341	2	1.803	4	.015	1	0	2	0	2	0	4
154		min	-176.683	9	.426	15	-.007	3	0	3	0	3	0	15
155		max	151.273	2	1.625	4	.015	1	0	2	0	2	0	2
156		min	-176.739	9	.384	15	-.007	3	0	3	0	3	0	15
157		max	151.206	2	1.447	4	.015	1	0	2	0	2	0	2
158		min	-176.795	9	.342	15	-.007	3	0	3	0	3	0	9
159		max	151.139	2	1.269	4	.015	1	0	2	0	2	0	10
160		min	-176.851	9	.3	15	-.007	3	0	3	0	3	0	1
161		max	151.072	2	1.091	4	.015	1	0	2	0	2	0	15
162		min	-176.907	9	.258	15	-.007	3	0	3	0	3	0	1
163		max	151.005	2	.913	4	.015	1	0	2	0	2	0	15
164		min	-176.963	9	.217	15	-.007	3	0	3	0	3	0	4
165		max	150.938	2	.735	4	.015	1	0	2	0	2	0	15
166		min	-177.019	9	.175	15	-.007	3	0	3	0	3	0	4
167		max	150.871	2	.557	4	.015	1	0	2	0	2	0	15
168		min	-177.075	9	.133	15	-.007	3	0	3	0	3	0	4
169	M8	max	150.804	2	.379	4	.015	1	0	2	0	2	0	15
170		min	-177.131	9	.091	15	-.007	3	0	3	0	3	-.001	4
171		max	150.737	2	.201	4	.015	1	0	2	0	2	0	15
172		min	-177.186	9	.049	15	-.007	3	0	3	0	3	-.001	4
173		max	150.67	2	.05	2	.015	1	0	2	0	2	0	15
174		min	-177.242	9	-.023	9	-.007	3	0	3	0	3	-.001	4
175		max	150.603	2	-.034	15	.015	1	0	2	0	2	0	15
176		min	-177.298	9	-.16	1	-.007	3	0	3	0	3	-.001	4
177		max	150.535	2	-.076	15	.015	1	0	2	0	2	0	15
178		min	-177.354	9	-.333	4	-.007	3	0	3	0	3	-.001	4
179		max	150.468	2	-.118	15	.015	1	0	2	0	2	0	15
180		min	-177.41	9	-.511	4	-.007	3	0	3	0	3	-.001	4
181		max	150.401	2	-.16	15	.015	1	0	2	0	2	0	15
182		min	-177.466	9	-.689	4	-.007	3	0	3	0	3	0	4
183		max	150.334	2	-.202	15	.015	1	0	2	0	2	0	15
184		min	-177.522	9	-.867	4	-.007	3	0	3	0	3	0	4
185		max	150.267	2	-.244	15	.015	1	0	2	0	2	0	15
186		min	-177.578	9	-1.045	4	-.007	3	0	3	0	3	0	4
187	M8	max	150.2	2	-.286	15	.015	1	0	2	0	2	0	15
188		min	-177.634	9	-1.223	4	-.007	3	0	3	0	3	0	4
189		max	150.133	2	-.327	15	.015	1	0	2	0	2	0	1
190		min	-177.69	9	-1.401	4	-.007	3	0	3	0	3	0	1
191		max	1774.79	1	0	1	.72	1	0	1	0	15	0	1
192		min	-375.047	3	0	1	-.197	3	0	1	0	1	0	1
193		max	1774.855	1	0	1	.72	1	0	1	0	1	0	1
194		min	-374.999	3	0	1	-.197	3	0	1	0	3	0	1
195		max	1774.919	1	0	1	.72	1	0	1	0	1	0	1
196		min	-374.95	3	0	1	-.197	3	0	1	0	3	0	1
197		max	1774.984	1	0	1	.72	1	0	1	0	1	0	1
198		min	-374.902	3	0	1	-.197	3	0	1	0	3	0	1
199		max	1775.049	1	0	1	.72	1	0	1	0	1	0	1
200		min	-374.853	3	0	1	-.197	3	0	1	0	3	0	1
201		max	1775.113	1	0	1	.72	1	0	1	0	1	0	1
202		min	-374.805	3	0	1	-.197	3	0	1	0	3	0	1
203		max	1775.178	1	0	1	.72	1	0	1	0	1	0	1
204	M8	min	-374.756	3	0	1	-.197	3	0	1	0	3	0	1
205		max	1775.243	1	0	1	.72	1	0	1	0	1	0	1
206		min	-374.708	3	0	1	-.197	3	0	1	0	3	0	1
207		max	1775.307	1	0	1	.72	1	0	1	0	1	0	1
208		min	-374.659	3	0	1	-.197	3	0	1	0	3	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1775.372	1	0	1	.72	1	0	1	0	1	0	1
210			min	-374.611	3	0	1	-.197	3	0	1	0	3	0	1
211		11	max	1775.437	1	0	1	.72	1	0	1	0	1	0	1
212			min	-374.562	3	0	1	-.197	3	0	1	0	3	0	1
213		12	max	1775.502	1	0	1	.72	1	0	1	0	1	0	1
214			min	-374.514	3	0	1	-.197	3	0	1	0	3	0	1
215		13	max	1775.566	1	0	1	.72	1	0	1	0	1	0	1
216			min	-374.465	3	0	1	-.197	3	0	1	0	3	0	1
217		14	max	1775.631	1	0	1	.72	1	0	1	0	1	0	1
218			min	-374.417	3	0	1	-.197	3	0	1	0	3	0	1
219		15	max	1775.696	1	0	1	.72	1	0	1	0	1	0	1
220			min	-374.368	3	0	1	-.197	3	0	1	0	3	0	1
221		16	max	1775.76	1	0	1	.72	1	0	1	0	1	0	1
222			min	-374.319	3	0	1	-.197	3	0	1	0	3	0	1
223		17	max	1775.825	1	0	1	.72	1	0	1	.001	1	0	1
224			min	-374.271	3	0	1	-.197	3	0	1	0	3	0	1
225		18	max	1775.89	1	0	1	.72	1	0	1	.001	1	0	1
226			min	-374.222	3	0	1	-.197	3	0	1	0	3	0	1
227		19	max	1775.955	1	0	1	.72	1	0	1	.001	1	0	1
228			min	-374.174	3	0	1	-.197	3	0	1	0	3	0	1
229	M10	1	max	446.511	1	.647	4	-.004	15	.001	1	0	2	0	1
230			min	-344.015	3	.155	15	-.13	1	0	3	0	3	0	1
231		2	max	446.607	1	.609	4	-.004	15	.001	1	0	2	0	15
232			min	-343.942	3	.146	15	-.13	1	0	3	0	3	0	4
233		3	max	446.704	1	.571	4	-.004	15	.001	1	0	2	0	15
234			min	-343.87	3	.137	15	-.13	1	0	3	0	3	0	4
235		4	max	446.8	1	.533	4	-.004	15	.001	1	0	2	0	15
236			min	-343.798	3	.128	15	-.13	1	0	3	0	3	0	4
237		5	max	446.897	1	.495	4	-.004	15	.001	1	0	2	0	15
238			min	-343.725	3	.12	15	-.13	1	0	3	0	1	0	4
239		6	max	446.993	1	.458	4	-.004	15	.001	1	0	15	0	15
240			min	-343.653	3	.111	15	-.13	1	0	3	0	1	0	4
241		7	max	447.089	1	.42	4	-.004	15	.001	1	0	15	0	15
242			min	-343.581	3	.102	15	-.13	1	0	3	0	1	0	4
243		8	max	447.186	1	.382	4	-.004	15	.001	1	0	15	0	15
244			min	-343.509	3	.093	15	-.13	1	0	3	0	1	0	4
245		9	max	447.282	1	.344	4	-.004	15	.001	1	0	15	0	15
246			min	-343.436	3	.084	15	-.13	1	0	3	0	1	0	4
247		10	max	447.378	1	.306	4	-.004	15	.001	1	0	15	0	15
248			min	-343.364	3	.075	15	-.13	1	0	3	0	1	0	4
249		11	max	447.475	1	.268	4	-.004	15	.001	1	0	15	0	15
250			min	-343.292	3	.066	15	-.13	1	0	3	0	1	0	4
251		12	max	447.571	1	.231	4	-.004	15	.001	1	0	15	0	15
252			min	-343.22	3	.057	15	-.13	1	0	3	0	1	0	4
253		13	max	447.667	1	.193	4	-.004	15	.001	1	0	15	0	15
254			min	-343.147	3	.048	15	-.13	1	0	3	0	1	0	4
255		14	max	447.764	1	.155	4	-.004	15	.001	1	0	15	0	15
256			min	-343.075	3	.023	1	-.13	1	0	3	0	1	0	4
257		15	max	447.86	1	.117	4	-.004	15	.001	1	0	15	0	15
258			min	-343.003	3	-.007	1	-.13	1	0	3	0	1	0	4
259		16	max	447.957	1	.095	3	-.004	15	.001	1	0	15	0	15
260			min	-342.93	3	-.036	1	-.13	1	0	3	0	1	0	4
261		17	max	448.053	1	.073	3	-.004	15	.001	1	0	15	0	15
262			min	-342.858	3	-.066	1	-.13	1	0	3	0	1	0	4
263		18	max	448.149	1	.051	3	-.004	15	.001	1	0	15	0	15
264			min	-342.786	3	-.095	1	-.13	1	0	3	0	1	0	4
265		19	max	448.246	1	.028	3	-.004	15	.001	1	0	15	0	15



Company : Schletter, Inc.  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-342.714	3	-.125	1	-.13	1	0	3	0	1	0	4
267	M11	1	max	31.731	10	1.816	4	.932	1	.001	1	0	3	0	4
268			min	-130.66	1	.427	15	.022	12	0	15	-.002	1	0	15
269		2	max	31.675	10	1.638	4	.932	1	.001	1	0	3	0	4
270			min	-130.727	1	.386	15	.022	12	0	15	-.002	1	0	15
271		3	max	31.619	10	1.46	4	.932	1	.001	1	0	3	0	2
272			min	-130.794	1	.344	15	.022	12	0	15	-.002	1	0	3
273		4	max	31.563	10	1.282	4	.932	1	.001	1	0	3	0	15
274			min	-130.861	1	.302	15	.022	12	0	15	-.002	1	0	4
275		5	max	31.507	10	1.104	4	.932	1	.001	1	0	3	0	15
276			min	-130.928	1	.26	15	.022	12	0	15	-.001	1	0	4
277		6	max	31.451	10	.926	4	.932	1	.001	1	0	3	0	15
278			min	-130.995	1	.218	15	.022	12	0	15	-.001	1	0	4
279		7	max	31.395	10	.748	4	.932	1	.001	1	0	3	0	15
280			min	-131.062	1	.176	15	.022	12	0	15	-.001	1	0	4
281		8	max	31.339	10	.57	4	.932	1	.001	1	0	3	0	15
282			min	-131.129	1	.135	15	.022	12	0	15	0	1	0	4
283		9	max	31.283	10	.392	4	.932	1	.001	1	0	3	0	15
284			min	-131.196	1	.093	15	.022	12	0	15	0	1	-.001	4
285		10	max	31.227	10	.214	4	.932	1	.001	1	0	3	0	15
286			min	-131.263	1	.051	15	.022	12	0	15	0	1	-.001	4
287		11	max	31.172	10	.049	2	.932	1	.001	1	0	3	0	15
288			min	-131.331	1	.003	3	.022	12	0	15	0	1	-.001	4
289		12	max	31.116	10	-.033	15	.932	1	.001	1	0	3	0	15
290			min	-131.398	1	-.142	4	.022	12	0	15	0	2	-.001	4
291		13	max	31.06	10	-.075	15	.932	1	.001	1	0	1	0	15
292			min	-131.465	1	-.32	4	.022	12	0	15	0	10	-.001	4
293		14	max	31.004	10	-.117	15	.932	1	.001	1	0	1	0	15
294			min	-131.532	1	-.498	4	.022	12	0	15	0	15	-.001	4
295		15	max	30.948	10	-.158	15	.932	1	.001	1	0	1	0	15
296			min	-131.599	1	-.676	4	.022	12	0	15	0	15	0	4
297		16	max	30.892	10	-.2	15	.932	1	.001	1	0	1	0	15
298			min	-131.666	1	-.854	4	.022	12	0	15	0	15	0	4
299		17	max	30.836	10	-.242	15	.932	1	.001	1	0	1	0	15
300			min	-131.733	1	-1.032	4	.022	12	0	15	0	15	0	4
301		18	max	30.78	10	-.284	15	.932	1	.001	1	.001	1	0	15
302			min	-131.8	1	-1.21	4	.022	12	0	15	0	15	0	4
303		19	max	30.724	10	-.326	15	.932	1	.001	1	.001	1	0	1
304			min	-131.867	1	-1.388	4	.022	12	0	15	0	15	0	1
305	M12	1	max	594.972	1	0	1	3.718	1	0	1	0	1	0	1
306			min	-106.776	3	0	1	.113	15	0	1	0	3	0	1
307		2	max	595.037	1	0	1	3.718	1	0	1	0	1	0	1
308			min	-106.728	3	0	1	.113	15	0	1	0	12	0	1
309		3	max	595.102	1	0	1	3.718	1	0	1	0	1	0	1
310			min	-106.679	3	0	1	.113	15	0	1	0	15	0	1
311		4	max	595.166	1	0	1	3.718	1	0	1	.001	1	0	1
312			min	-106.63	3	0	1	.113	15	0	1	0	15	0	1
313		5	max	595.231	1	0	1	3.718	1	0	1	.001	1	0	1
314			min	-106.582	3	0	1	.113	15	0	1	0	15	0	1
315		6	max	595.296	1	0	1	3.718	1	0	1	.002	1	0	1
316			min	-106.533	3	0	1	.113	15	0	1	0	15	0	1
317		7	max	595.361	1	0	1	3.718	1	0	1	.002	1	0	1
318			min	-106.485	3	0	1	.113	15	0	1	0	15	0	1
319		8	max	595.425	1	0	1	3.718	1	0	1	.002	1	0	1
320			min	-106.436	3	0	1	.113	15	0	1	0	15	0	1
321		9	max	595.49	1	0	1	3.718	1	0	1	.003	1	0	1
322			min	-106.388	3	0	1	.113	15	0	1	0	15	0	1





Company : Schletter, Inc.  
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Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-117.97	1	-154.408	3	-75.136	1	0	1	-.143	1	-.007	3
381	M5	1	max	259.127	1	1093.598	3	-.085	10	0	.004	1	.019	3
382		min	7.128	12	-1437.035	1	-24.937	1	0	3	0	10	-.029	1
383		2	max	259.199	1	1093.395	3	-.085	10	0	0	2	.282	1
384		min	7.164	12	-1437.304	1	-24.937	1	0	3	-.002	3	-.218	3
385		3	max	309.298	1	10.668	9	2.446	3	0	0	10	.588	1
386		min	-29.776	3	-74.129	3	-.162	2	0	1	-.007	3	-.451	3
387		4	max	309.37	1	10.444	9	2.446	3	0	0	10	.591	1
388		min	-29.722	3	-74.331	3	-.162	2	0	1	-.006	3	-.434	3
389		5	max	309.442	1	10.219	9	2.446	3	0	0	10	.594	1
390		min	-29.668	3	-74.533	3	-.162	2	0	1	-.006	3	-.418	3
391		6	max	309.515	1	9.994	9	2.446	3	0	0	10	.597	1
392		min	-29.614	3	-74.736	3	-.162	2	0	1	-.005	3	-.402	3
393		7	max	309.587	1	9.769	9	2.446	3	0	0	10	.6	1
394		min	-29.559	3	-74.938	3	-.162	2	0	1	-.005	1	-.386	3
395		8	max	309.659	1	9.545	9	2.446	3	0	0	10	.603	1
396		min	-29.505	3	-75.14	3	-.162	2	0	1	-.004	1	-.37	3
397		9	max	309.731	1	9.32	9	2.446	3	0	0	10	.606	1
398		min	-29.451	3	-75.342	3	-.162	2	0	1	-.004	1	-.353	3
399		10	max	309.804	1	9.095	9	2.446	3	0	0	10	.61	1
400		min	-29.397	3	-75.545	3	-.162	2	0	1	-.003	1	-.337	3
401		11	max	309.876	1	8.87	9	2.446	3	0	0	10	.613	1
402		min	-29.343	3	-75.747	3	-.162	2	0	1	-.003	1	-.321	3
403		12	max	309.948	1	8.646	9	2.446	3	0	0	10	.616	1
404		min	-29.288	3	-75.949	3	-.162	2	0	1	-.003	1	-.304	3
405		13	max	310.02	1	8.421	9	2.446	3	0	0	10	.62	1
406		min	-29.234	3	-76.152	3	-.162	2	0	1	-.002	1	-.288	3
407		14	max	310.093	1	8.196	9	2.446	3	0	0	15	.623	1
408		min	-29.18	3	-76.354	3	-.162	2	0	1	-.002	1	-.271	3
409		15	max	310.165	1	7.971	9	2.446	3	0	0	15	.627	1
410		min	-29.126	3	-76.556	3	-.162	2	0	1	-.002	1	-.254	3
411		16	max	249.019	2	48.417	10	2.426	3	0	0	3	.631	1
412		min	-108.13	3	-145.294	3	-.159	2	0	15	-.001	1	-.237	3
413		17	max	249.092	2	48.192	10	2.426	3	0	0	3	.655	1
414		min	-108.075	3	-145.497	3	-.159	2	0	15	0	1	-.206	3
415		18	max	-7.51	12	1596.696	1	2.251	1	0	.001	3	.315	1
416		min	-259.87	1	-508.696	3	-.013	10	0	1	0	2	-.097	3
417		19	max	-7.474	12	1596.426	1	2.251	1	0	.002	3	.014	3
418		min	-259.798	1	-508.899	3	-.013	10	0	1	0	2	-.031	1
419	M9	1	max	117.925	1	330.541	3	98.03	1	0	-.004	15	.015	1
420		min	3.615	15	-434.101	1	3.115	15	0	1	-.144	1	-.009	3
421		2	max	117.997	1	330.339	3	98.03	1	0	-.003	12	.109	1
422		min	3.636	15	-434.371	1	3.115	15	0	1	-.123	1	-.081	3
423		3	max	134.342	1	7.156	9	68.407	1	0	.001	3	.201	1
424		min	-5.606	3	-22.377	3	1.136	12	0	15	-.1	1	-.151	3
425		4	max	134.414	1	6.931	9	68.407	1	0	.002	3	.201	1
426		min	-5.552	3	-22.579	3	1.136	12	0	15	-.085	1	-.146	3
427		5	max	134.487	1	6.706	9	68.407	1	0	.002	3	.201	1
428		min	-5.498	3	-22.781	3	1.136	12	0	15	-.07	1	-.141	3
429		6	max	134.559	1	6.481	9	68.407	1	0	.002	3	.201	1
430		min	-5.443	3	-22.984	3	1.136	12	0	15	-.056	1	-.137	3
431		7	max	134.631	1	6.256	9	68.407	1	0	.003	3	.201	1
432		min	-5.389	3	-23.186	3	1.136	12	0	15	-.041	1	-.131	3
433		8	max	134.703	1	6.032	9	68.407	1	0	.003	3	.201	1
434		min	-5.335	3	-23.388	3	1.136	12	0	15	-.026	1	-.126	3
435		9	max	134.776	1	5.807	9	68.407	1	0	.003	3	.201	1
436		min	-5.281	3	-23.59	3	1.136	12	0	15	-.011	1	-.121	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	134.848	1	5.582	9	68.407	1	0	1	.004	1	.201	1
438			min	-5.227	3	-23.793	3	1.136	12	0	15	0	10	-.116	3
439		11	max	134.92	1	5.357	9	68.407	1	0	1	.019	1	.201	1
440			min	-5.172	3	-23.995	3	1.136	12	0	15	0	15	-.111	3
441		12	max	134.993	1	5.133	9	68.407	1	0	1	.033	1	.201	1
442			min	-5.118	3	-24.197	3	1.136	12	0	15	.001	15	-.106	3
443		13	max	135.065	1	4.908	9	68.407	1	0	1	.048	1	.202	1
444			min	-5.064	3	-24.4	3	1.136	12	0	15	.001	15	-.101	3
445		14	max	135.137	1	4.683	9	68.407	1	0	1	.063	1	.202	1
446			min	-5.01	3	-24.602	3	1.136	12	0	15	.002	15	-.095	3
447		15	max	135.209	1	4.458	9	68.407	1	0	1	.078	1	.203	1
448			min	-4.956	3	-24.804	3	1.136	12	0	15	.002	15	-.09	3
449		16	max	66.679	2	7.66	10	69.282	1	0	15	.095	1	.204	1
450			min	-33.362	3	-89.224	1	1.161	12	0	1	.003	15	-.084	3
451		17	max	66.751	2	7.436	10	69.282	1	0	15	.11	1	.223	1
452			min	-33.308	3	-89.494	1	1.161	12	0	1	.003	15	-.073	3
453		18	max	-3.631	15	484.039	1	72.897	1	0	1	.125	1	.12	1
454			min	-117.825	1	-154.205	3	1.348	12	0	3	.004	15	-.04	3
455		19	max	-3.609	15	483.769	1	72.897	1	0	1	.141	1	.016	1
456			min	-117.753	1	-154.407	3	1.348	12	0	3	.004	15	-.007	3
457	M13	1	max	98.217	1	433.506	1	-3.615	15	.015	1	.144	1	0	1
458			min	3.115	15	-330.531	3	-117.912	1	-.009	3	.004	15	0	3
459		2	max	98.217	1	305.748	1	-2.772	15	.015	1	.045	1	.266	3
460			min	3.115	15	-233.057	3	-90.383	1	-.009	3	.001	15	-.349	1
461		3	max	98.217	1	177.99	1	-1.93	15	.015	1	0	3	.44	3
462			min	3.115	15	-135.583	3	-62.854	1	-.009	3	-.027	1	-.577	1
463		4	max	98.217	1	50.232	1	-1.087	15	.015	1	-.001	12	.522	3
464			min	3.115	15	-38.109	3	-35.325	1	-.009	3	-.073	1	-.685	1
465		5	max	98.217	1	59.366	3	-.245	15	.015	1	-.002	12	.512	3
466			min	3.115	15	-77.526	1	-7.796	1	-.009	3	-.094	1	-.672	1
467		6	max	98.217	1	156.84	3	19.733	1	.015	1	-.002	12	.41	3
468			min	3.115	15	-205.284	1	.315	12	-.009	3	-.088	1	-.539	1
469		7	max	98.217	1	254.314	3	47.262	1	.015	1	-.001	12	.216	3
470			min	3.115	15	-333.042	1	1.137	12	-.009	3	-.056	1	-.285	1
471		8	max	98.217	1	351.788	3	74.791	1	.015	1	.001	1	.09	1
472			min	3.115	15	-460.8	1	1.959	12	-.009	3	0	12	-.07	3
473		9	max	98.217	1	449.262	3	102.32	1	.015	1	.085	1	.586	1
474			min	3.115	15	-588.558	1	2.781	12	-.009	3	.002	12	-.449	3
475		10	max	98.217	1	546.736	3	129.849	1	.011	2	.195	1	1.202	1
476			min	3.115	15	-716.316	1	3.602	12	-.015	1	.005	12	-.919	3
477		11	max	73.344	1	588.558	1	-2.702	12	.009	3	.081	1	.586	1
478			min	2.263	15	-449.262	3	-101.772	1	-.015	1	0	12	-.449	3
479		12	max	73.344	1	460.8	1	-1.88	12	.009	3	0	10	.09	1
480			min	2.263	15	-351.788	3	-74.243	1	-.015	1	-.002	3	-.07	3
481		13	max	73.344	1	333.042	1	-1.058	12	.009	3	-.002	15	.216	3
482			min	2.263	15	-254.314	3	-46.714	1	-.015	1	-.059	1	-.285	1
483		14	max	73.344	1	205.284	1	-.236	12	.009	3	-.003	15	.41	3
484			min	2.263	15	-156.84	3	-19.185	1	-.015	1	-.09	1	-.539	1
485		15	max	73.344	1	77.526	1	8.344	1	.009	3	-.003	15	.512	3
486			min	2.263	15	-59.365	3	.263	15	-.015	1	-.095	1	-.672	1
487		16	max	73.344	1	38.109	3	35.873	1	.009	3	-.002	12	.522	3
488			min	2.263	15	-50.232	1	1.105	15	-.015	1	-.074	1	-.685	1
489		17	max	73.344	1	135.583	3	63.402	1	.009	3	0	12	.44	3
490			min	2.263	15	-177.99	1	1.948	15	-.015	1	-.028	1	-.577	1
491		18	max	73.344	1	233.057	3	90.931	1	.009	3	.045	1	.266	3
492			min	2.263	15	-305.748	1	2.79	15	-.015	1	.001	15	-.349	1
493		19	max	73.344	1	330.531	3	118.46	1	.009	3	.144	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	2.263	15	-433.506	1	3.632	15	-.015	1	.004	15	0	3
495	M16	1	max	-1.348	12	484.398	1	-3.609	15	.007	3	.141	1	0	1
496			min	-72.668	1	-154.422	3	-117.763	1	-.016	1	.004	15	0	3
497		2	max	-1.348	12	341.628	1	-2.767	15	.007	3	.043	1	.124	3
498			min	-72.668	1	-108.987	3	-90.234	1	-.016	1	.001	15	-.39	1
499		3	max	-1.348	12	198.859	1	-1.924	15	.007	3	0	12	.206	3
500			min	-72.668	1	-63.552	3	-62.705	1	-.016	1	-.029	1	-.645	1
501		4	max	-1.348	12	56.089	1	-1.082	15	.007	3	-.002	15	.244	3
502			min	-72.668	1	-18.116	3	-35.176	1	-.016	1	-.075	1	-.766	1
503		5	max	-1.348	12	27.319	3	-.239	15	.007	3	-.003	15	.24	3
504			min	-72.668	1	-86.68	1	-7.647	1	-.016	1	-.096	1	-.751	1
505		6	max	-1.348	12	72.754	3	19.882	1	.007	3	-.003	15	.193	3
506			min	-72.668	1	-229.45	1	.403	12	-.016	1	-.09	1	-.602	1
507		7	max	-1.348	12	118.189	3	47.411	1	.007	3	-.002	15	.103	3
508			min	-72.668	1	-372.219	1	1.225	12	-.016	1	-.058	1	-.318	1
509		8	max	-1.348	12	163.624	3	74.94	1	.007	3	0	2	.101	1
510			min	-72.668	1	-514.989	1	2.047	12	-.016	1	-.001	3	-.03	3
511		9	max	-1.348	12	209.059	3	102.469	1	.007	3	.083	1	.655	1
512			min	-72.668	1	-657.758	1	2.869	12	-.016	1	.001	12	-.206	3
513		10	max	-2.314	15	-17.629	15	129.998	1	0	15	.193	1	1.344	1
514			min	-74.936	1	-800.528	1	-5.66	3	-.016	1	.006	12	-.425	3
515		11	max	-2.314	15	657.758	1	-2.961	12	.016	1	.084	1	.655	1
516			min	-74.936	1	-209.059	3	-102.251	1	-.007	3	.002	12	-.206	3
517		12	max	-2.314	15	514.989	1	-2.139	12	.016	1	0	2	.101	1
518			min	-74.936	1	-163.624	3	-74.722	1	-.007	3	0	3	-.03	3
519		13	max	-2.314	15	372.219	1	-1.317	12	.016	1	-.002	12	.103	3
520			min	-74.936	1	-118.189	3	-47.193	1	-.007	3	-.057	1	-.318	1
521		14	max	-2.314	15	229.45	1	-.495	12	.016	1	-.003	12	.193	3
522			min	-74.936	1	-72.754	3	-19.664	1	-.007	3	-.089	1	-.602	1
523		15	max	-2.314	15	86.68	1	7.865	1	.016	1	-.003	12	.24	3
524			min	-74.936	1	-27.319	3	.246	15	-.007	3	-.094	1	-.751	1
525		16	max	-2.314	15	18.116	3	35.394	1	.016	1	-.002	12	.244	3
526			min	-74.936	1	-56.089	1	1.088	15	-.007	3	-.074	1	-.766	1
527		17	max	-2.314	15	63.552	3	62.923	1	.016	1	0	12	.206	3
528			min	-74.936	1	-198.859	1	1.931	15	-.007	3	-.028	1	-.645	1
529		18	max	-2.314	15	108.987	3	90.452	1	.016	1	.045	1	.124	3
530			min	-74.936	1	-341.628	1	2.773	15	-.007	3	.001	15	-.39	1
531		19	max	-2.314	15	154.422	3	117.981	1	.016	1	.143	1	0	1
532			min	-74.936	1	-484.398	1	3.615	12	-.007	3	.004	15	0	3
533	M15	1	max	0	10	2.956	4	.02	3	0	1	0	1	0	1
534			min	-26.929	1	0	10	-.029	1	0	3	0	3	0	1
535		2	max	0	10	2.628	4	.02	3	0	1	0	1	0	10
536			min	-27.001	1	0	10	-.029	1	0	3	0	3	-.001	4
537		3	max	0	10	2.299	4	.02	3	0	1	0	1	0	10
538			min	-27.073	1	0	10	-.029	1	0	3	0	3	-.003	4
539		4	max	0	10	1.971	4	.02	3	0	1	0	1	0	10
540			min	-27.145	1	0	10	-.029	1	0	3	0	3	-.004	4
541		5	max	0	10	1.642	4	.02	3	0	1	0	1	0	10
542			min	-27.217	1	0	10	-.029	1	0	3	0	3	-.005	4
543		6	max	0	10	1.314	4	.02	3	0	1	0	1	0	10
544			min	-27.289	1	0	10	-.029	1	0	3	0	3	-.005	4
545		7	max	0	10	.985	4	.02	3	0	1	0	3	0	10
546			min	-27.361	1	0	10	-.029	1	0	3	0	1	-.006	4
547		8	max	0	10	.657	4	.02	3	0	1	0	3	0	10
548			min	-27.433	1	0	10	-.029	1	0	3	0	1	-.006	4
549		9	max	0	10	.328	4	.02	3	0	1	0	3	0	10
550			min	-27.505	1	0	10	-.029	1	0	3	0	1	-.006	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	10	0	1	.02	3	0	1	0	3	0	10
552			min	-27.577	1	0	1	-.029	1	0	3	0	1	-.007	4
553		11	max	0	10	0	10	.02	3	0	1	0	3	0	10
554			min	-27.649	1	-.328	4	-.029	1	0	3	0	1	-.006	4
555		12	max	0	10	0	10	.02	3	0	1	0	3	0	10
556			min	-27.721	1	-.657	4	-.029	1	0	3	0	1	-.006	4
557		13	max	0	10	0	10	.02	3	0	1	0	3	0	10
558			min	-27.793	1	-.985	4	-.029	1	0	3	0	1	-.006	4
559		14	max	0	10	0	10	.02	3	0	1	0	3	0	10
560			min	-27.865	1	-1.314	4	-.029	1	0	3	0	1	-.005	4
561		15	max	0	10	0	10	.02	3	0	1	0	3	0	10
562			min	-27.937	1	-1.642	4	-.029	1	0	3	0	1	-.005	4
563		16	max	0	10	0	10	.02	3	0	1	0	3	0	10
564			min	-28.009	1	-1.971	4	-.029	1	0	3	0	1	-.004	4
565		17	max	0	10	0	10	.02	3	0	1	0	3	0	10
566			min	-28.081	1	-2.299	4	-.029	1	0	3	0	1	-.003	4
567		18	max	0	10	0	10	.02	3	0	1	0	3	0	10
568			min	-28.152	1	-2.628	4	-.029	1	0	3	0	1	-.001	4
569		19	max	0	10	0	10	.02	3	0	1	0	3	0	1
570			min	-28.224	1	-2.956	4	-.029	1	0	3	0	1	0	1
571	M16A	1	max	-.796	10	2.956	4	.018	1	0	3	0	3	0	1
572			min	-31.691	1	.695	15	-.008	3	0	1	0	1	0	1
573		2	max	-.736	10	2.628	4	.018	1	0	3	0	3	0	15
574			min	-31.619	1	.618	15	-.008	3	0	1	0	1	-.001	4
575		3	max	-.676	10	2.299	4	.018	1	0	3	0	3	0	15
576			min	-31.547	1	.54	15	-.008	3	0	1	0	1	-.003	4
577		4	max	-.616	10	1.971	4	.018	1	0	3	0	3	0	15
578			min	-31.475	1	.463	15	-.008	3	0	1	0	1	-.004	4
579		5	max	-.556	10	1.642	4	.018	1	0	3	0	3	-.001	15
580			min	-31.403	1	.386	15	-.008	3	0	1	0	1	-.005	4
581		6	max	-.496	10	1.314	4	.018	1	0	3	0	3	-.001	15
582			min	-31.331	1	.309	15	-.008	3	0	1	0	1	-.005	4
583		7	max	-.436	10	.985	4	.018	1	0	3	0	3	-.001	15
584			min	-31.259	1	.232	15	-.008	3	0	1	0	1	-.006	4
585		8	max	-.377	10	.657	4	.018	1	0	3	0	3	-.001	15
586			min	-31.187	1	.154	15	-.008	3	0	1	0	1	-.006	4
587		9	max	-.317	10	.328	4	.018	1	0	3	0	3	-.002	15
588			min	-31.115	1	.077	15	-.008	3	0	1	0	1	-.006	4
589		10	max	-.257	10	0	1	.018	1	0	3	0	3	-.002	15
590			min	-31.043	1	0	1	-.008	3	0	1	0	1	-.007	4
591		11	max	-.197	10	-.077	15	.018	1	0	3	0	3	-.002	15
592			min	-30.971	1	-.328	4	-.008	3	0	1	0	1	-.006	4
593		12	max	-.137	10	-.154	15	.018	1	0	3	0	3	-.001	15
594			min	-30.899	1	-.657	4	-.008	3	0	1	0	1	-.006	4
595		13	max	-.077	10	-.232	15	.018	1	0	3	0	1	-.001	15
596			min	-30.827	1	-.985	4	-.008	3	0	1	0	13	-.006	4
597		14	max	-.017	10	-.309	15	.018	1	0	3	0	1	-.001	15
598			min	-30.755	1	-1.314	4	-.008	3	0	1	0	3	-.005	4
599		15	max	.043	10	-.386	15	.018	1	0	3	0	1	-.001	15
600			min	-30.683	1	-1.642	4	-.008	3	0	1	0	3	-.005	4
601		16	max	.103	10	-.463	15	.018	1	0	3	0	1	0	15
602			min	-30.611	1	-1.971	4	-.008	3	0	1	0	3	-.004	4
603		17	max	.163	10	-.54	15	.018	1	0	3	0	1	0	15
604			min	-30.539	1	-2.299	4	-.008	3	0	1	0	3	-.003	4
605		18	max	.223	10	-.618	15	.018	1	0	3	0	1	0	15
606			min	-30.467	1	-2.628	4	-.008	3	0	1	0	3	-.001	4
607		19	max	.283	10	-.695	15	.018	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-30.395	1	-2.956	4	-.008	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.014	1	-3.239e-5	15	NC	3	NC	3	
2			min	-.003	3	-.005	3	0	3	-1.053e-3	1	4988.179	2	2210.585	1	
3			2	max	.003	1	.006	2	.013	1	-3.11e-5	15	NC	3	NC	3
4				min	-.002	3	-.004	3	0	3	-1.011e-3	1	5401.385	2	2396.361	1
5			3	max	.003	1	.005	2	.012	1	-2.981e-5	15	NC	3	NC	3
6				min	-.002	3	-.004	3	0	3	-9.698e-4	1	5885.723	2	2614.915	1
7			4	max	.003	1	.005	2	.01	1	-2.852e-5	15	NC	3	NC	3
8				min	-.002	3	-.004	3	0	3	-9.283e-4	1	6457.248	2	2874.23	1
9			5	max	.003	1	.004	2	.009	1	-2.723e-5	15	NC	3	NC	3
10				min	-.002	3	-.004	3	0	3	-8.868e-4	1	7137.056	2	3184.965	1
11		6	max	.002	1	.004	2	.008	1	-2.595e-5	15	NC	1	NC	3	
12			min	-.002	3	-.004	3	0	3	-8.453e-4	1	7953.307	2	3561.598	1	
13		7	max	.002	1	.003	2	.007	1	-2.466e-5	15	NC	1	NC	2	
14			min	-.002	3	-.004	3	0	3	-8.038e-4	1	8944.305	2	4024.219	1	
15		8	max	.002	1	.003	2	.007	1	-2.337e-5	15	NC	1	NC	2	
16			min	-.002	3	-.003	3	0	3	-7.623e-4	1	NC	1	4601.38	1	
17		9	max	.002	1	.003	2	.006	1	-2.208e-5	15	NC	1	NC	2	
18			min	-.001	3	-.003	3	0	3	-7.208e-4	1	NC	1	5334.811	1	
19		10	max	.002	1	.002	2	.005	1	-2.079e-5	15	NC	1	NC	2	
20			min	-.001	3	-.003	3	0	3	-6.793e-4	1	NC	1	6287.544	1	
21		11	max	.001	1	.002	2	.004	1	-1.95e-5	15	NC	1	NC	2	
22			min	-.001	3	-.003	3	0	3	-6.378e-4	1	NC	1	7558.64	1	
23		12	max	.001	1	.002	2	.003	1	-1.822e-5	15	NC	1	NC	2	
24			min	-.001	3	-.002	3	0	3	-5.963e-4	1	NC	1	9311.485	1	
25		13	max	.001	1	.001	2	.003	1	-1.693e-5	15	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-5.548e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.002	1	-1.564e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-5.133e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	-1.435e-5	15	NC	1	NC	1	
30			min	0	3	-.001	3	0	3	-4.718e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.306e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-4.303e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.178e-5	15	NC	1	NC	1	
34			min	0	3	0	3	0	3	-3.888e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.043e-5	12	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.473e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-7.456e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.058e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.39e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.487e-6	12	NC	1	NC	1	
41			2	max	0	1	0	2	0	12	1.773e-4	1	NC	1	NC	1
42				min	0	10	0	3	0	1	5.27e-6	12	NC	1	NC	1
43			3	max	0	1	0	2	0	12	2.156e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	-.001	1	6.556e-6	15	NC	1	NC	1
45			4	max	0	1	0	2	0	12	2.538e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	-.001	1	7.742e-6	15	NC	1	NC	1
47			5	max	0	1	0	2	0	12	2.921e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	-.001	1	8.928e-6	15	NC	1	NC	1
49			6	max	0	1	0	2	0	3	3.303e-4	1	NC	1	NC	1
50				min	0	10	-.003	3	-.001	1	1.011e-5	15	NC	1	NC	1
51		7	max	0	1	0	2	0	3	3.686e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	10	-.004	3	0	1	1.13e-5	15	NC	1	NC	1
53		8	max	0	1	.001	2	0	3	4.068e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	1.249e-5	15	NC	1	NC	1
55		9	max	0	1	.002	2	0	3	4.451e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	2	1.367e-5	15	NC	1	NC	1
57		10	max	0	1	.002	2	0	1	4.833e-4	1	NC	1	NC	1
58			min	0	10	-.005	3	0	15	1.486e-5	15	NC	1	NC	1
59		11	max	0	1	.002	2	.001	1	5.216e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	15	1.605e-5	15	NC	1	NC	1
61		12	max	0	1	.003	2	.002	1	5.599e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	15	1.723e-5	15	NC	1	NC	1
63		13	max	0	1	.004	1	.003	1	5.981e-4	1	NC	1	NC	1
64			min	0	10	-.006	3	0	15	1.842e-5	15	NC	1	NC	1
65		14	max	.001	1	.005	1	.003	1	6.364e-4	1	NC	3	NC	1
66			min	0	10	-.007	3	0	15	1.96e-5	15	9841.508	1	NC	1
67		15	max	.001	1	.006	1	.004	1	6.746e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	15	2.079e-5	15	8221.128	1	NC	1
69		16	max	.001	1	.007	1	.005	1	7.129e-4	1	NC	3	NC	2
70			min	0	10	-.007	3	0	15	2.198e-5	15	6981.609	1	9104.213	1
71		17	max	.001	1	.008	1	.006	1	7.511e-4	1	NC	3	NC	2
72			min	0	10	-.007	3	0	15	2.316e-5	15	6020.409	1	7925.564	1
73		18	max	.001	1	.009	1	.007	1	7.894e-4	1	NC	3	NC	2
74			min	0	10	-.007	3	0	15	2.435e-5	15	5266.44	1	7065.435	1
75		19	max	.001	1	.01	1	.007	1	8.276e-4	1	NC	3	NC	2
76			min	0	10	-.007	3	0	15	2.553e-5	15	4669.686	1	6428.849	1
77	M4	1	max	.003	1	.007	2	0	15	-2.836e-5	15	NC	1	NC	2
78			min	0	3	-.005	3	-.005	1	-9.288e-4	1	NC	1	3605.231	1
79		2	max	.003	1	.007	2	0	15	-2.836e-5	15	NC	1	NC	2
80			min	0	3	-.005	3	-.005	1	-9.288e-4	1	NC	1	3933.009	1
81		3	max	.003	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
82			min	0	3	-.004	3	-.004	1	-9.288e-4	1	NC	1	4323.115	1
83		4	max	.002	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
84			min	0	3	-.004	3	-.004	1	-9.288e-4	1	NC	1	4791.991	1
85		5	max	.002	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
86			min	0	3	-.004	3	-.004	1	-9.288e-4	1	NC	1	5362.034	1
87		6	max	.002	1	.005	2	0	15	-2.836e-5	15	NC	1	NC	2
88			min	0	3	-.004	3	-.003	1	-9.288e-4	1	NC	1	6064.394	1
89		7	max	.002	1	.005	2	0	15	-2.836e-5	15	NC	1	NC	2
90			min	0	3	-.003	3	-.003	1	-9.288e-4	1	NC	1	6943.447	1
91		8	max	.002	1	.004	2	0	15	-2.836e-5	15	NC	1	NC	2
92			min	0	3	-.003	3	-.002	1	-9.288e-4	1	NC	1	8064.175	1
93		9	max	.002	1	.004	2	0	15	-2.836e-5	15	NC	1	NC	2
94			min	0	3	-.003	3	-.002	1	-9.288e-4	1	NC	1	9524.879	1
95		10	max	.001	1	.004	2	0	15	-2.836e-5	15	NC	1	NC	1
96			min	0	3	-.003	3	-.002	1	-9.288e-4	1	NC	1	NC	1
97		11	max	.001	1	.003	2	0	15	-2.836e-5	15	NC	1	NC	1
98			min	0	3	-.002	3	-.001	1	-9.288e-4	1	NC	1	NC	1
99		12	max	.001	1	.003	2	0	15	-2.836e-5	15	NC	1	NC	1
100			min	0	3	-.002	3	-.001	1	-9.288e-4	1	NC	1	NC	1
101		13	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-9.288e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
104			min	0	3	-.001	3	0	1	-9.288e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-9.288e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-2.836e-5	15	NC	1	NC	1
108			min	0	3	0	3	0	1	-9.288e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-2.836e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-9.288e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-2.836e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-9.288e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.836e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.288e-4	1	NC	1	NC	1
115	M6	1	max	.011	1	.019	2	.004	1	2.321e-4	1	NC	3	NC	2
116			min	-.009	3	-.013	3	-.002	3	4.679e-6	10	1547.634	2	7634.685	1
117		2	max	.01	1	.018	2	.004	1	2.175e-4	1	NC	3	NC	2
118			min	-.008	3	-.012	3	-.002	3	3.867e-6	10	1650.376	2	8282.118	1
119		3	max	.009	1	.017	2	.003	1	2.029e-4	1	NC	3	NC	2
120			min	-.008	3	-.012	3	-.002	3	3.056e-6	10	1767.405	2	9051.421	1
121		4	max	.009	1	.016	2	.003	1	1.883e-4	1	NC	3	NC	2
122			min	-.007	3	-.011	3	-.001	3	2.244e-6	10	1901.568	2	9973.637	1
123		5	max	.008	1	.015	2	.003	1	1.737e-4	1	NC	3	NC	1
124			min	-.007	3	-.01	3	-.001	3	1.432e-6	10	2056.527	2	NC	1
125		6	max	.008	1	.013	2	.002	1	1.591e-4	1	NC	3	NC	1
126			min	-.006	3	-.01	3	-.001	3	6.205e-7	10	2237.077	2	NC	1
127		7	max	.007	1	.012	2	.002	1	1.445e-4	1	NC	3	NC	1
128			min	-.006	3	-.009	3	-.001	3	-1.912e-7	10	2449.609	2	NC	1
129		8	max	.006	1	.011	2	.002	1	1.299e-4	1	NC	3	NC	1
130			min	-.005	3	-.008	3	0	3	-1.003e-6	10	2702.846	2	NC	1
131		9	max	.006	1	.01	2	.002	1	1.154e-4	1	NC	3	NC	1
132			min	-.005	3	-.008	3	0	3	-1.815e-6	10	3008.998	2	NC	1
133		10	max	.005	1	.009	2	.001	1	1.093e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	0	3	-2.626e-6	10	3385.701	2	NC	1
135		11	max	.005	1	.008	2	.001	1	1.059e-4	3	NC	3	NC	1
136			min	-.004	3	-.006	3	0	3	-3.438e-6	10	3859.413	2	NC	1
137		12	max	.004	1	.007	2	0	1	1.026e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	0	3	-8.007e-6	2	4471.705	2	NC	1
139		13	max	.004	1	.006	2	0	1	9.921e-5	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-1.266e-5	2	5291.859	2	NC	1
141		14	max	.003	1	.005	2	0	1	9.584e-5	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-1.731e-5	2	6444.573	2	NC	1
143		15	max	.002	1	.004	2	0	1	9.248e-5	3	NC	3	NC	1
144			min	-.002	3	-.003	3	0	3	-2.196e-5	2	8179.23	2	NC	1
145		16	max	.002	1	.003	2	0	1	8.912e-5	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-2.662e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	8.575e-5	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-3.127e-5	2	NC	1	NC	1
149		18	max	0	1	0	2	0	1	8.239e-5	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-3.592e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	7.903e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.057e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.817e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-3.58e-5	3	NC	1	NC	1
155		2	max	0	9	.001	1	0	3	1.54e-5	2	NC	1	NC	1
156			min	0	2	-.001	3	0	2	-2.732e-5	3	NC	1	NC	1
157		3	max	0	9	.003	1	0	3	1.39e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-1.884e-5	3	NC	1	NC	1
159		4	max	0	9	.004	1	0	3	1.421e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	2	-1.036e-5	3	NC	1	NC	1
161		5	max	0	9	.005	1	0	3	1.451e-5	1	NC	3	NC	1
162			min	0	2	-.005	3	0	2	-1.887e-6	3	8893.567	1	NC	1
163		6	max	0	9	.007	1	0	3	1.481e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	3.806e-7	15	7039.404	1	NC	1
165		7	max	0	9	.008	1	0	3	1.512e-5	1	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	4.299e-7	15	5777.992	1	NC	1
167		8	max	0	9	.009	1	0	3	2.354e-5	3	NC	3	NC	1
168			min	0	2	-.009	3	0	1	-1.227e-6	2	4859.964	1	NC	1
169		9	max	0	9	.011	1	.001	3	3.202e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	-3.997e-6	2	4160.559	1	NC	1
171		10	max	.001	9	.013	1	.001	3	4.05e-5	3	NC	3	NC	1
172			min	0	2	-.012	3	0	1	-6.768e-6	2	3610.218	1	NC	1
173		11	max	.001	9	.015	1	.001	3	4.897e-5	3	NC	3	NC	1
174			min	0	2	-.013	3	-.001	1	-9.539e-6	2	3166.881	1	NC	1
175		12	max	.001	9	.016	1	.001	3	5.745e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-1.231e-5	2	2803.441	1	NC	1
177		13	max	.001	9	.018	1	.001	3	6.593e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.001	1	-1.508e-5	2	2501.514	1	NC	1
179		14	max	.001	9	.02	1	.001	3	7.441e-5	3	NC	3	NC	1
180			min	-.001	2	-.015	3	-.001	1	-1.785e-5	2	2248.098	1	NC	1
181		15	max	.002	9	.023	1	.001	3	8.288e-5	3	NC	3	NC	1
182			min	-.001	2	-.016	3	-.002	1	-2.062e-5	2	2033.68	1	NC	1
183		16	max	.002	9	.025	1	.001	3	9.136e-5	3	NC	3	NC	1
184			min	-.001	2	-.017	3	-.002	1	-2.339e-5	2	1851.11	1	NC	1
185		17	max	.002	9	.027	1	.001	3	9.984e-5	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-2.616e-5	2	1694.891	1	NC	1
187		18	max	.002	9	.029	1	.001	3	1.083e-4	3	NC	3	NC	1
188			min	-.002	2	-.018	3	-.002	1	-2.893e-5	2	1560.727	1	NC	1
189		19	max	.002	9	.032	1	0	3	1.168e-4	3	NC	3	NC	1
190			min	-.002	2	-.019	3	-.002	1	-3.17e-5	2	1445.214	1	NC	1
191	M8	1	max	.008	1	.023	2	.002	1	-3.06e-7	10	NC	1	NC	2
192			min	-.002	3	-.014	3	0	3	-1.095e-4	1	NC	1	8506.352	1
193		2	max	.008	1	.021	2	.002	1	-3.06e-7	10	NC	1	NC	2
194			min	-.002	3	-.013	3	0	3	-1.095e-4	1	NC	1	9274.25	1
195		3	max	.008	1	.02	2	.002	1	-3.06e-7	10	NC	1	NC	1
196			min	-.002	3	-.013	3	0	3	-1.095e-4	1	NC	1	NC	1
197		4	max	.007	1	.019	2	.002	1	-3.06e-7	10	NC	1	NC	1
198			min	-.001	3	-.012	3	0	3	-1.095e-4	1	NC	1	NC	1
199		5	max	.007	1	.018	2	.002	1	-3.06e-7	10	NC	1	NC	1
200			min	-.001	3	-.011	3	0	3	-1.095e-4	1	NC	1	NC	1
201		6	max	.006	1	.016	2	.001	1	-3.06e-7	10	NC	1	NC	1
202			min	-.001	3	-.01	3	0	3	-1.095e-4	1	NC	1	NC	1
203		7	max	.006	1	.015	2	.001	1	-3.06e-7	10	NC	1	NC	1
204			min	-.001	3	-.01	3	0	3	-1.095e-4	1	NC	1	NC	1
205		8	max	.005	1	.014	2	.001	1	-3.06e-7	10	NC	1	NC	1
206			min	-.001	3	-.009	3	0	3	-1.095e-4	1	NC	1	NC	1
207		9	max	.005	1	.013	2	0	1	-3.06e-7	10	NC	1	NC	1
208			min	0	3	-.008	3	0	3	-1.095e-4	1	NC	1	NC	1
209		10	max	.004	1	.011	2	0	1	-3.06e-7	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.095e-4	1	NC	1	NC	1
211		11	max	.004	1	.01	2	0	1	-3.06e-7	10	NC	1	NC	1
212			min	0	3	-.006	3	0	3	-1.095e-4	1	NC	1	NC	1
213		12	max	.003	1	.009	2	0	1	-3.06e-7	10	NC	1	NC	1
214			min	0	3	-.006	3	0	3	-1.095e-4	1	NC	1	NC	1
215		13	max	.003	1	.008	2	0	1	-3.06e-7	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.095e-4	1	NC	1	NC	1
217		14	max	.002	1	.006	2	0	1	-3.06e-7	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.095e-4	1	NC	1	NC	1
219		15	max	.002	1	.005	2	0	1	-3.06e-7	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.095e-4	1	NC	1	NC	1
221		16	max	.001	1	.004	2	0	1	-3.06e-7	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.095e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-3.06e-7	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.095e-4	1	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-3.06e-7	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.095e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-3.06e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.095e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.006	2	0	3	9.271e-4	1	NC	3	NC	1
230			min	-.003	3	-.005	3	-.002	1	-1.411e-4	3	4997.773	2	NC	1
231		2	max	.003	1	.006	2	0	3	8.789e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.002	1	-1.376e-4	3	5396.919	2	NC	1
233		3	max	.003	1	.005	2	0	3	8.306e-4	1	NC	3	NC	1
234			min	-.002	3	-.004	3	-.002	1	-1.34e-4	3	5862.203	2	NC	1
235		4	max	.003	1	.005	2	0	3	7.823e-4	1	NC	3	NC	1
236			min	-.002	3	-.004	3	-.001	1	-1.305e-4	3	6407.97	2	NC	1
237		5	max	.003	1	.004	2	0	3	7.341e-4	1	NC	3	NC	1
238			min	-.002	3	-.004	3	-.001	1	-1.27e-4	3	7052.923	2	NC	1
239		6	max	.002	1	.004	2	0	3	6.858e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	-.001	1	-1.235e-4	3	7821.83	2	NC	1
241		7	max	.002	1	.003	2	0	3	6.376e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	-.001	1	-1.199e-4	3	8748.1	2	NC	1
243		8	max	.002	1	.003	2	0	3	5.893e-4	1	NC	1	NC	1
244			min	-.002	3	-.003	3	0	1	-1.164e-4	3	9877.773	2	NC	1
245		9	max	.002	1	.003	2	0	3	5.411e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-1.129e-4	3	NC	1	NC	1
247		10	max	.002	1	.002	2	0	3	4.928e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-1.094e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.446e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-1.058e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	3.963e-4	1	NC	1	NC	1
252			min	0	3	-.002	3	0	1	-1.023e-4	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	3.481e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-9.879e-5	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	2.998e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-9.526e-5	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.515e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-9.173e-5	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.033e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-8.821e-5	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.55e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-8.468e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.068e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-8.116e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	5.852e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-7.763e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	3.537e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.812e-5	1	NC	1	NC	1
269		2	max	0	1	0	2	0	2	2.501e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-9.509e-5	1	NC	1	NC	1
271		3	max	0	1	0	2	0	2	1.465e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-1.621e-4	1	NC	1	NC	1
273		4	max	0	1	0	2	0	10	4.281e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-2.291e-4	1	NC	1	NC	1
275		5	max	0	1	0	2	0	10	-4.547e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	-.001	1	-2.96e-4	1	NC	1	NC	1
277		6	max	0	1	0	2	0	15	-1.097e-5	12	NC	1	NC	1
278			min	0	10	-.003	3	-.002	1	-3.63e-4	1	NC	1	NC	1
279		7	max	0	1	.001	2	0	15	-1.321e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-.004	3	-.003	1	-4.3e-4	1	NC	1	NC	1
281		8	max	0	1	.001	2	0	15	-1.53e-5	15	NC	1	NC	1
282			min	0	10	-.005	3	-.003	1	-4.97e-4	1	NC	1	NC	1
283		9	max	0	1	.002	2	0	15	-1.739e-5	15	NC	1	NC	1
284			min	0	10	-.005	3	-.004	1	-5.639e-4	1	NC	1	NC	1
285		10	max	0	1	.002	2	0	15	-1.948e-5	15	NC	1	NC	2
286			min	0	10	-.006	3	-.005	1	-6.309e-4	1	NC	1	8479.713	1
287		11	max	0	1	.003	2	0	15	-2.157e-5	15	NC	1	NC	2
288			min	0	10	-.006	3	-.007	1	-6.979e-4	1	NC	1	7064.286	1
289		12	max	0	1	.003	1	0	15	-2.366e-5	15	NC	1	NC	2
290			min	0	10	-.006	3	-.008	1	-7.649e-4	1	NC	1	6035.479	1
291		13	max	0	1	.004	1	0	15	-2.575e-5	15	NC	1	NC	2
292			min	0	10	-.007	3	-.009	1	-8.319e-4	1	NC	1	5266.47	1
293		14	max	.001	1	.005	1	0	15	-2.784e-5	15	NC	3	NC	2
294			min	0	10	-.007	3	-.01	1	-8.988e-4	1	9555.086	1	4679.448	1
295		15	max	.001	1	.006	1	0	15	-2.992e-5	15	NC	3	NC	2
296			min	0	10	-.007	3	-.011	1	-9.658e-4	1	8056.581	1	4224.592	1
297		16	max	.001	1	.007	1	0	15	-3.201e-5	15	NC	3	NC	2
298			min	0	10	-.007	3	-.012	1	-1.033e-3	1	6891.527	1	3868.926	1
299		17	max	.001	1	.008	1	0	15	-3.41e-5	15	NC	3	NC	2
300			min	0	10	-.007	3	-.013	1	-1.1e-3	1	5976.43	1	3590.096	1
301		18	max	.001	1	.009	1	0	15	-3.619e-5	15	NC	3	NC	3
302			min	0	10	-.007	3	-.014	1	-1.167e-3	1	5251.311	1	3372.736	1
303		19	max	.001	1	.01	1	0	15	-3.828e-5	15	NC	3	NC	3
304			min	0	10	-.007	3	-.014	1	-1.234e-3	1	4672.758	1	3206.305	1
305	M12	1	max	.003	1	.007	2	.012	1	1.056e-3	1	NC	1	NC	3
306			min	0	3	-.005	3	0	15	3.317e-5	15	NC	1	1630.624	1
307		2	max	.003	1	.007	2	.011	1	1.056e-3	1	NC	1	NC	3
308			min	0	3	-.005	3	0	15	3.317e-5	15	NC	1	1778.315	1
309		3	max	.003	1	.006	2	.01	1	1.056e-3	1	NC	1	NC	3
310			min	0	3	-.004	3	0	15	3.317e-5	15	NC	1	1954.118	1
311		4	max	.002	1	.006	2	.009	1	1.056e-3	1	NC	1	NC	3
312			min	0	3	-.004	3	0	15	3.317e-5	15	NC	1	2165.441	1
313		5	max	.002	1	.006	2	.008	1	1.056e-3	1	NC	1	NC	3
314			min	0	3	-.004	3	0	15	3.317e-5	15	NC	1	2422.379	1
315		6	max	.002	1	.005	2	.007	1	1.056e-3	1	NC	1	NC	3
316			min	0	3	-.004	3	0	15	3.317e-5	15	NC	1	2738.972	1
317		7	max	.002	1	.005	2	.006	1	1.056e-3	1	NC	1	NC	3
318			min	0	3	-.003	3	0	15	3.317e-5	15	NC	1	3135.219	1
319		8	max	.002	1	.004	2	.005	1	1.056e-3	1	NC	1	NC	3
320			min	0	3	-.003	3	0	15	3.317e-5	15	NC	1	3640.407	1
321		9	max	.002	1	.004	2	.004	1	1.056e-3	1	NC	1	NC	2
322			min	0	3	-.003	3	0	15	3.317e-5	15	NC	1	4298.834	1
323		10	max	.001	1	.004	2	.004	1	1.056e-3	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	3.317e-5	15	NC	1	5180.15	1
325		11	max	.001	1	.003	2	.003	1	1.056e-3	1	NC	1	NC	2
326			min	0	3	-.002	3	0	15	3.317e-5	15	NC	1	6399.073	1
327		12	max	.001	1	.003	2	.002	1	1.056e-3	1	NC	1	NC	2
328			min	0	3	-.002	3	0	15	3.317e-5	15	NC	1	8155.737	1
329		13	max	0	1	.002	2	.002	1	1.056e-3	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	3.317e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	.001	1	1.056e-3	1	NC	1	NC	1
332			min	0	3	-.001	3	0	15	3.317e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	1.056e-3	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	3.317e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	1.056e-3	1	NC	1	NC	1
336			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	1.056e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.056e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.056e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.317e-5	15	NC	1	NC	1
343	M1	1	max	.005	3	.021	3	0	3	2.156e-2	1	NC	1	NC	1
344			min	-.006	2	-.032	1	-.004	1	-1.635e-2	3	NC	1	NC	1
345		2	max	.005	3	.011	3	0	3	1.038e-2	1	NC	4	NC	2
346			min	-.006	2	-.017	1	-.01	1	-8.093e-3	3	3104.714	1	8288.182	1
347		3	max	.005	3	.002	3	0	3	1.472e-5	3	NC	5	NC	2
348			min	-.006	2	-.003	1	-.014	1	-5.917e-4	1	1605.999	1	5023.944	1
349		4	max	.005	3	.009	1	0	3	1.76e-5	3	NC	5	NC	2
350			min	-.006	2	-.005	3	-.016	1	-4.857e-4	1	1136.684	1	4154.701	1
351		5	max	.005	3	.019	1	0	3	2.048e-5	3	NC	5	NC	2
352			min	-.006	2	-.012	3	-.016	1	-3.798e-4	1	911.244	1	3985.909	1
353		6	max	.005	3	.027	1	0	3	2.336e-5	3	NC	5	NC	2
354			min	-.006	2	-.016	3	-.015	1	-2.738e-4	1	783.948	1	4259.159	1
355		7	max	.005	3	.034	1	0	3	2.624e-5	3	NC	5	NC	2
356			min	-.006	2	-.02	3	-.013	1	-1.679e-4	1	707.012	1	5059.841	1
357		8	max	.005	3	.038	1	0	3	2.912e-5	3	NC	5	NC	2
358			min	-.006	2	-.023	3	-.011	1	-6.195e-5	1	660.573	1	6915.026	1
359		9	max	.005	3	.041	1	0	3	4.4e-5	1	NC	5	NC	1
360			min	-.006	2	-.024	3	-.008	1	1.587e-6	15	635.366	1	NC	1
361		10	max	.005	3	.042	1	0	3	1.499e-4	1	NC	5	NC	1
362			min	-.006	2	-.024	3	-.004	1	4.832e-6	15	627.133	1	NC	1
363		11	max	.005	3	.041	1	0	3	2.559e-4	1	NC	5	NC	1
364			min	-.006	2	-.023	3	-.001	1	8.078e-6	15	634.596	1	NC	1
365		12	max	.005	3	.038	1	.002	1	3.618e-4	1	NC	5	NC	2
366			min	-.006	2	-.021	3	0	15	1.132e-5	15	658.936	1	8171.488	1
367		13	max	.005	3	.033	1	.005	1	4.678e-4	1	NC	5	NC	2
368			min	-.006	2	-.019	3	0	15	1.457e-5	15	704.282	1	5652.809	1
369		14	max	.005	3	.027	1	.006	1	5.737e-4	1	NC	5	NC	2
370			min	-.006	2	-.015	3	0	15	1.781e-5	15	779.661	1	4626.143	1
371		15	max	.005	3	.018	1	.007	1	6.797e-4	1	NC	5	NC	2
372			min	-.006	2	-.01	3	0	15	2.106e-5	15	904.406	1	4255.928	1
373		16	max	.005	3	.008	1	.007	1	7.577e-4	1	NC	5	NC	2
374			min	-.006	2	-.005	3	0	15	2.346e-5	15	1124.594	1	4382.33	1
375		17	max	.005	3	.002	3	.005	1	1.714e-4	1	NC	5	NC	2
376			min	-.006	2	-.004	1	0	15	5.888e-6	15	1578.485	1	5254.013	1
377		18	max	.005	3	.009	3	.002	1	1.2e-2	1	NC	4	NC	2
378			min	-.006	2	-.019	1	0	15	-3.838e-3	3	3042.672	1	8616.532	1
379		19	max	.005	3	.016	3	0	3	2.407e-2	1	NC	1	NC	1
380			min	-.006	2	-.034	1	-.003	1	-7.775e-3	3	NC	1	NC	1
381	M5	1	max	.013	3	.064	3	0	3	4.19e-7	1	NC	1	NC	1
382			min	-.02	2	-.095	1	-.005	1	3.587e-8	15	NC	1	NC	1
383		2	max	.013	3	.035	3	.001	3	3.497e-5	3	NC	5	NC	1
384			min	-.02	2	-.051	1	-.005	1	-9.635e-5	1	1044.076	1	NC	1
385		3	max	.013	3	.008	3	.002	3	6.892e-5	3	NC	5	NC	1
386			min	-.02	2	-.009	1	-.004	1	-1.911e-4	1	537.747	1	NC	1
387		4	max	.013	3	.026	1	.002	3	6.85e-5	3	NC	5	NC	1
388			min	-.02	2	-.015	3	-.003	1	-1.78e-4	1	379.816	1	NC	1
389		5	max	.013	3	.057	1	.002	3	6.807e-5	3	NC	15	NC	1
390			min	-.02	2	-.033	3	-.003	1	-1.65e-4	1	303.936	1	NC	1
391		6	max	.013	3	.082	1	.002	3	6.764e-5	3	NC	15	NC	1
392			min	-.02	2	-.047	3	-.003	1	-1.519e-4	1	261.039	1	NC	1
393		7	max	.013	3	.101	1	.003	3	6.721e-5	3	NC	15	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.02	2	-.058	3	-.002	1	-1.389e-4	1	235.049	1	NC	1
395		8	max	.013	3	.115	1	.003	3	6.678e-5	3	9833.078	15	NC	1
396			min	-.02	2	-.065	3	-.002	1	-1.258e-4	1	219.282	1	NC	1
397		9	max	.013	3	.123	1	.002	3	6.635e-5	3	9485.444	15	NC	1
398			min	-.02	2	-.069	3	-.002	1	-1.128e-4	1	210.618	1	NC	1
399		10	max	.013	3	.126	1	.002	3	6.593e-5	3	9388.568	15	NC	1
400			min	-.02	2	-.069	3	-.002	1	-9.975e-5	1	207.614	1	NC	1
401		11	max	.013	3	.123	1	.002	3	6.55e-5	3	9525.389	15	NC	1
402			min	-.02	2	-.067	3	-.002	1	-8.67e-5	1	209.83	1	NC	1
403		12	max	.013	3	.115	1	.002	3	6.507e-5	3	9915.42	15	NC	1
404			min	-.02	2	-.061	3	-.002	1	-7.365e-5	1	217.642	1	NC	1
405		13	max	.013	3	.101	1	.002	3	6.464e-5	3	NC	15	NC	1
406			min	-.02	2	-.053	3	-.002	1	-6.06e-5	1	232.408	1	NC	1
407		14	max	.013	3	.081	1	.001	3	6.421e-5	3	NC	15	NC	1
408			min	-.02	2	-.042	3	-.002	1	-4.756e-5	1	257.114	1	NC	1
409		15	max	.013	3	.055	1	.001	3	6.378e-5	3	NC	15	NC	1
410			min	-.02	2	-.029	3	-.002	1	-3.451e-5	1	298.18	1	NC	1
411		16	max	.013	3	.024	1	0	3	6.179e-5	3	NC	5	NC	1
412			min	-.02	2	-.013	3	-.002	1	-3.111e-5	2	370.995	1	NC	1
413		17	max	.013	3	.005	3	0	3	2.245e-5	3	NC	5	NC	1
414			min	-.02	2	-.013	1	-.002	1	-1.94e-4	1	522.446	1	NC	1
415		18	max	.013	3	.025	3	0	3	1.092e-5	3	NC	5	NC	1
416			min	-.02	2	-.057	1	-.002	1	-9.947e-5	1	1011.93	1	NC	1
417		19	max	.013	3	.046	3	0	3	0	5	NC	1	NC	1
418			min	-.02	2	-.104	1	-.003	1	-8.39e-8	4	NC	1	NC	1
419	M9	1	max	.005	3	.021	3	0	3	1.635e-2	3	NC	1	NC	1
420			min	-.006	2	-.032	1	-.006	1	-2.156e-2	1	NC	1	NC	1
421		2	max	.005	3	.011	3	0	3	8.111e-3	3	NC	4	NC	2
422			min	-.006	2	-.017	1	-.001	1	-1.065e-2	1	3105.543	1	9770.334	1
423		3	max	.005	3	.002	3	.002	1	4.98e-5	1	NC	5	NC	2
424			min	-.006	2	-.003	1	0	3	1.741e-6	15	1606.44	1	6085.253	1
425		4	max	.005	3	.009	1	.004	1	1.022e-5	3	NC	5	NC	2
426			min	-.006	2	-.005	3	0	3	-3.872e-5	1	1136.997	1	5171.686	1
427		5	max	.005	3	.019	1	.004	1	1.556e-6	3	NC	5	NC	2
428			min	-.006	2	-.012	3	0	3	-1.272e-4	1	911.488	1	5145.839	1
429		6	max	.005	3	.027	1	.003	1	-4.887e-6	12	NC	5	NC	2
430			min	-.006	2	-.017	3	-.001	3	-2.158e-4	1	784.148	1	5810.403	1
431		7	max	.005	3	.034	1	.001	1	-9.206e-6	15	NC	5	NC	2
432			min	-.006	2	-.02	3	-.001	3	-3.043e-4	1	707.182	1	7598.818	1
433		8	max	.005	3	.038	1	0	2	-1.194e-5	15	NC	5	NC	1
434			min	-.006	2	-.023	3	-.002	3	-3.928e-4	1	660.722	1	NC	1
435		9	max	.005	3	.041	1	0	10	-1.468e-5	15	NC	5	NC	1
436			min	-.006	2	-.024	3	-.004	1	-4.813e-4	1	635.5	1	NC	1
437		10	max	.005	3	.042	1	0	15	-1.742e-5	15	NC	5	NC	1
438			min	-.006	2	-.024	3	-.006	1	-5.698e-4	1	627.255	1	NC	1
439		11	max	.005	3	.041	1	0	15	-2.015e-5	15	NC	5	NC	2
440			min	-.006	2	-.023	3	-.009	1	-6.584e-4	1	634.709	1	8652.558	1
441		12	max	.005	3	.038	1	0	15	-2.289e-5	15	NC	5	NC	2
442			min	-.006	2	-.021	3	-.012	1	-7.469e-4	1	659.044	1	5739.114	1
443		13	max	.005	3	.033	1	0	15	-2.563e-5	15	NC	5	NC	2
444			min	-.006	2	-.019	3	-.014	1	-8.354e-4	1	704.386	1	4506.538	1
445		14	max	.005	3	.027	1	0	15	-2.836e-5	15	NC	5	NC	2
446			min	-.006	2	-.015	3	-.015	1	-9.239e-4	1	779.763	1	3942.263	1
447		15	max	.005	3	.018	1	0	15	-3.11e-5	15	NC	5	NC	2
448			min	-.006	2	-.01	3	-.015	1	-1.012e-3	1	904.512	1	3778.637	1
449		16	max	.005	3	.008	1	0	15	-3.313e-5	15	NC	5	NC	2
450			min	-.006	2	-.005	3	-.014	1	-1.079e-3	1	1124.71	1	3999.451	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	15	-1.841e-5	15	NC	5	NC	2
452			min	-.006	2	-.004	1	-.012	1	-6.182e-4	1	1578.637	1	4890	1
453		18	max	.005	3	.009	3	0	15	3.837e-3	3	NC	4	NC	2
454			min	-.006	2	-.019	1	-.008	1	-1.223e-2	1	3042.952	1	8137.276	1
455		19	max	.005	3	.016	3	0	3	7.774e-3	3	NC	1	NC	1
456			min	-.006	2	-.034	1	-.002	1	-2.407e-2	1	NC	1	NC	1
457	M13	1	max	.006	1	.021	3	.005	3	3.756e-3	3	NC	1	NC	1
458			min	0	3	-.032	1	-.006	2	-5.702e-3	1	NC	1	NC	1
459		2	max	.006	1	.2	3	.045	1	4.549e-3	3	NC	5	NC	2
460			min	0	3	-.268	1	0	10	-6.931e-3	1	865.013	1	4069.812	1
461		3	max	.006	1	.346	3	.116	1	5.343e-3	3	NC	5	NC	3
462			min	0	3	-.46	1	.004	15	-8.161e-3	1	476.016	1	1691.257	1
463		4	max	.006	1	.437	3	.176	1	6.136e-3	3	NC	15	NC	3
464			min	0	3	-.581	1	.006	15	-9.39e-3	1	371.506	1	1129.863	1
465		5	max	.006	1	.463	3	.205	1	6.93e-3	3	NC	15	NC	3
466			min	0	3	-.616	1	.006	15	-1.062e-2	1	349.43	1	970.948	1
467		6	max	.006	1	.424	3	.196	1	7.724e-3	3	NC	5	NC	3
468			min	0	3	-.566	1	.006	15	-1.185e-2	1	381.829	1	1017.105	1
469		7	max	.006	1	.334	3	.15	1	8.517e-3	3	NC	5	NC	3
470			min	0	3	-.449	1	.005	15	-1.308e-2	1	488.637	1	1321.602	1
471		8	max	.005	1	.218	3	.08	1	9.311e-3	3	NC	5	NC	3
472			min	0	3	-.298	1	0	10	-1.431e-2	1	767.529	1	2400.165	1
473		9	max	.005	1	.112	3	.013	9	1.01e-2	3	NC	5	NC	1
474			min	0	3	-.159	1	-.007	10	-1.554e-2	1	1608.27	1	NC	1
475		10	max	.005	1	.064	3	.013	3	1.09e-2	3	NC	4	NC	1
476			min	0	3	-.095	1	-.02	2	-1.677e-2	1	3203.778	1	NC	1
477		11	max	.005	1	.112	3	.017	1	1.01e-2	3	NC	5	NC	2
478			min	0	3	-.159	1	-.007	10	-1.554e-2	1	1608.271	1	9443.499	1
479		12	max	.005	1	.218	3	.088	1	9.311e-3	3	NC	5	NC	3
480			min	0	3	-.298	1	0	10	-1.431e-2	1	767.529	1	2209.453	1
481		13	max	.005	1	.334	3	.158	1	8.518e-3	3	NC	5	NC	3
482			min	0	3	-.449	1	.005	15	-1.308e-2	1	488.637	1	1250.557	1
483		14	max	.005	1	.424	3	.205	1	7.724e-3	3	NC	5	NC	3
484			min	0	3	-.566	1	.006	15	-1.185e-2	1	381.829	1	973.394	1
485		15	max	.005	1	.463	3	.214	1	6.931e-3	3	NC	15	NC	3
486			min	0	3	-.616	1	.007	15	-1.062e-2	1	349.43	1	934.177	1
487		16	max	.005	1	.437	3	.183	1	6.138e-3	3	NC	15	NC	3
488			min	0	3	-.581	1	.006	15	-9.39e-3	1	371.506	1	1088.972	1
489		17	max	.005	1	.346	3	.121	1	5.344e-3	3	NC	5	NC	3
490			min	0	3	-.46	1	.004	15	-8.16e-3	1	476.016	1	1627.084	1
491		18	max	.005	1	.2	3	.048	1	4.551e-3	3	NC	5	NC	2
492			min	0	3	-.268	1	0	10	-6.931e-3	1	865.013	1	3882.1	1
493		19	max	.004	1	.021	3	.005	3	3.758e-3	3	NC	1	NC	1
494			min	0	3	-.032	1	-.006	2	-5.701e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.016	3	.005	3	5.908e-3	1	NC	1	NC	1
496			min	0	3	-.034	1	-.006	2	-2.816e-3	3	NC	1	NC	1
497		2	max	.002	1	.101	3	.048	1	7.212e-3	1	NC	5	NC	2
498			min	0	3	-.297	1	0	10	-3.385e-3	3	774.705	1	3801.527	1
499		3	max	.002	1	.171	3	.121	1	8.516e-3	1	NC	5	NC	3
500			min	0	3	-.513	1	.004	15	-3.953e-3	3	426.351	1	1610.729	1
501		4	max	.002	1	.215	3	.183	1	9.82e-3	1	NC	15	NC	3
502			min	0	3	-.647	1	.006	15	-4.522e-3	3	332.789	1	1084.577	1
503		5	max	.002	1	.228	3	.213	1	1.112e-2	1	NC	15	NC	3
504			min	0	3	-.686	1	.007	15	-5.09e-3	3	313.084	1	934.804	1
505		6	max	.002	1	.212	3	.203	1	1.243e-2	1	NC	5	NC	3
506			min	0	3	-.63	1	.006	15	-5.659e-3	3	342.248	1	978.823	1
507		7	max	.002	1	.171	3	.156	1	1.373e-2	1	NC	5	NC	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-499	1	.005	15	-6.227e-3	3	438.317	1	1266.14	1
509	8	max	.002	1	.117	3	.085	1	1.504e-2	1	NC	5	NC	3
510		min	0	3	-.33	1	-.001	10	-6.796e-3	3	689.687	1	2268.44	1
511	9	max	.003	1	.068	3	.015	3	1.634e-2	1	NC	5	NC	1
512		min	0	3	-.174	1	-.007	10	-7.364e-3	3	1452.478	1	NC	1
513	10	max	.003	1	.046	3	.013	3	1.764e-2	1	NC	4	NC	1
514		min	0	3	-.104	1	-.02	2	-7.933e-3	3	2921.211	1	NC	1
515	11	max	.003	1	.068	3	.014	3	1.634e-2	1	NC	5	NC	1
516		min	0	3	-.175	1	-.007	10	-7.364e-3	3	1452.478	1	NC	1
517	12	max	.003	1	.117	3	.082	1	1.504e-2	1	NC	5	NC	3
518		min	0	3	-.33	1	-.001	10	-6.795e-3	3	689.687	1	2338.358	1
519	13	max	.003	1	.171	3	.152	1	1.373e-2	1	NC	5	NC	3
520		min	0	3	-499	1	.005	15	-6.226e-3	3	438.317	1	1296.074	1
521	14	max	.003	1	.212	3	.199	1	1.243e-2	1	NC	5	NC	3
522		min	0	3	-.63	1	.006	15	-5.657e-3	3	342.248	1	1000.07	1
523	15	max	.003	1	.228	3	.208	1	1.113e-2	1	NC	15	NC	3
524		min	0	3	-.686	1	.007	15	-5.089e-3	3	313.084	1	955.475	1
525	16	max	.003	1	.215	3	.178	1	9.822e-3	1	NC	15	NC	3
526		min	0	3	-.647	1	.006	15	-4.52e-3	3	332.789	1	1111.239	1
527	17	max	.003	1	.171	3	.118	1	8.519e-3	1	NC	5	NC	3
528		min	0	3	-.513	1	.004	15	-3.951e-3	3	426.351	1	1659.408	1
529	18	max	.003	1	.101	3	.046	1	7.215e-3	1	NC	5	NC	2
530		min	0	3	-.297	1	0	10	-3.382e-3	3	774.706	1	3967.152	1
531	19	max	.003	1	.016	3	.005	3	5.911e-3	1	NC	1	NC	1
532		min	0	3	-.034	1	-.006	2	-2.813e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	2.738e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-1.039e-4	2	NC	1	NC	1
535	2	max	0	1	-.007	15	.001	1	7.566e-4	3	NC	5	NC	1
536		min	0	10	-.029	4	0	3	-8.764e-4	1	3692.493	4	NC	1
537	3	max	0	1	-.013	15	.004	1	1.239e-3	3	7993.466	15	NC	1
538		min	0	10	-.057	4	-.003	3	-1.664e-3	1	1878.984	4	NC	1
539	4	max	0	1	-.019	15	.008	1	1.722e-3	3	5483.984	15	NC	2
540		min	0	10	-.083	4	-.006	3	-2.452e-3	1	1289.092	4	8992.134	1
541	5	max	0	1	-.025	15	.014	1	2.205e-3	3	4279.209	15	NC	4
542		min	0	10	-.106	4	-.009	3	-3.24e-3	1	1005.892	4	5842.892	1
543	6	max	0	1	-.03	15	.019	1	2.687e-3	3	3601.406	15	NC	4
544		min	0	10	-.126	4	-.013	3	-4.028e-3	1	846.564	4	4225.222	1
545	7	max	0	1	-.033	15	.025	1	3.17e-3	3	3193.798	15	NC	4
546		min	0	10	-.142	4	-.017	3	-4.816e-3	1	750.75	4	3286.488	1
547	8	max	0	1	-.036	15	.031	1	3.653e-3	3	2949.171	15	NC	4
548		min	0	10	-.154	4	-.021	3	-5.604e-3	1	693.247	4	2699.453	1
549	9	max	0	1	-.038	15	.037	1	4.136e-3	3	2817.499	15	NC	4
550		min	0	10	-.161	4	-.025	3	-6.392e-3	1	662.295	4	2316.574	1
551	10	max	0	1	-.038	15	.041	1	4.618e-3	3	2775.844	15	NC	4
552		min	0	10	-.164	4	-.028	3	-7.18e-3	1	652.504	4	2064.212	1
553	11	max	0	1	-.038	15	.044	1	5.101e-3	3	2817.499	15	NC	4
554		min	0	10	-.161	4	-.03	3	-7.968e-3	1	662.295	4	1903.822	1
555	12	max	0	1	-.036	15	.046	1	5.584e-3	3	2949.171	15	NC	5
556		min	0	10	-.154	4	-.032	3	-8.756e-3	1	693.247	4	1816.197	1
557	13	max	0	1	-.033	15	.045	1	6.067e-3	3	3193.798	15	NC	5
558		min	0	10	-.143	4	-.031	3	-9.544e-3	1	750.75	4	1795.455	1
559	14	max	0	1	-.03	15	.043	1	6.549e-3	3	3601.406	15	NC	4
560		min	0	10	-.127	4	-.029	3	-1.033e-2	1	846.564	4	1849	1
561	15	max	0	1	-.025	15	.037	1	7.032e-3	3	4279.209	15	NC	4
562		min	0	10	-.107	4	-.025	3	-1.112e-2	1	1005.892	4	2005.198	1
563	16	max	0	1	-.02	15	.028	1	7.515e-3	3	5483.984	15	NC	4
564		min	0	10	-.084	4	-.019	3	-1.191e-2	1	1289.092	4	2341.561	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	1	-.013	15	.016	1	7.997e-3	3	7993.466	15	NC	4
566		min	0	10	-.058	4	-.011	3	-1.27e-2	1	1878.984	4	3101.655	1
567	18	max	0	1	-.007	15	.002	9	8.48e-3	3	NC	5	NC	4
568		min	0	10	-.03	4	-.005	2	-1.348e-2	1	3692.493	4	5518.058	1
569	19	max	0	1	.004	3	.015	3	8.963e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.021	2	-1.427e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.005	3	3.022e-3	3	NC	1	NC	1
572		min	0	1	-.002	1	-.006	2	-4.482e-3	1	NC	1	NC	1
573	2	max	0	10	-.007	15	.006	1	2.884e-3	3	NC	5	NC	2
574		min	0	1	-.029	4	0	10	-4.261e-3	1	3692.493	4	9383.262	1
575	3	max	0	10	-.013	15	.015	1	2.745e-3	3	7993.466	15	NC	3
576		min	0	1	-.057	4	-.004	3	-4.04e-3	1	1878.984	4	5305.363	1
577	4	max	0	10	-.019	15	.022	1	2.606e-3	3	5483.984	15	NC	4
578		min	0	1	-.083	4	-.007	3	-3.819e-3	1	1289.092	4	4031.939	1
579	5	max	0	10	-.025	15	.026	1	2.467e-3	3	4279.209	15	NC	4
580		min	0	1	-.106	4	-.009	3	-3.597e-3	1	1005.892	4	3479.006	1
581	6	max	0	10	-.03	15	.029	1	2.328e-3	3	3601.406	15	NC	4
582		min	0	1	-.126	4	-.011	3	-3.376e-3	1	846.564	4	3236.066	1
583	7	max	0	10	-.033	15	.03	1	2.19e-3	3	3193.798	15	NC	4
584		min	0	1	-.142	4	-.011	3	-3.155e-3	1	750.75	4	3174.319	1
585	8	max	0	10	-.036	15	.029	1	2.051e-3	3	2949.171	15	NC	4
586		min	0	1	-.154	4	-.011	3	-2.934e-3	1	693.247	4	3249.456	1
587	9	max	0	10	-.038	15	.028	1	1.912e-3	3	2817.499	15	NC	4
588		min	0	1	-.161	4	-.011	3	-2.713e-3	1	662.295	4	3454.991	1
589	10	max	0	10	-.038	15	.025	1	1.773e-3	3	2775.844	15	NC	4
590		min	0	1	-.163	4	-.01	3	-2.492e-3	1	652.504	4	3811.293	1
591	11	max	0	10	-.038	15	.022	1	1.634e-3	3	2817.499	15	NC	4
592		min	0	1	-.161	4	-.008	3	-2.271e-3	1	662.295	4	4369.838	1
593	12	max	0	10	-.036	15	.018	1	1.496e-3	3	2949.171	15	NC	3
594		min	0	1	-.154	4	-.007	3	-2.049e-3	1	693.247	4	5232.902	1
595	13	max	0	10	-.033	15	.014	1	1.357e-3	3	3193.798	15	NC	2
596		min	0	1	-.142	4	-.005	3	-1.828e-3	1	750.75	4	6604.069	1
597	14	max	0	10	-.03	15	.01	1	1.218e-3	3	3601.406	15	NC	2
598		min	0	1	-.126	4	-.004	3	-1.607e-3	1	846.564	4	8921.917	1
599	15	max	0	10	-.025	15	.007	1	1.079e-3	3	4279.209	15	NC	1
600		min	0	1	-.106	4	-.002	3	-1.386e-3	1	1005.892	4	NC	1
601	16	max	0	10	-.019	15	.004	1	9.403e-4	3	5483.984	15	NC	1
602		min	0	1	-.083	4	-.001	3	-1.165e-3	1	1289.092	4	NC	1
603	17	max	0	10	-.013	15	.001	1	8.015e-4	3	7993.466	15	NC	1
604		min	0	1	-.057	4	0	3	-9.437e-4	1	1878.984	4	NC	1
605	18	max	0	10	-.007	15	0	4	6.627e-4	3	NC	5	NC	1
606		min	0	1	-.029	4	0	2	-7.226e-4	1	3692.493	4	NC	1
607	19	max	0	1	0	1	0	1	5.239e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.057e-4	2	NC	1	NC	1



**Anchor Designer™**  
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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.