



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	Height = 1550 mm
Width =	1050 mm	Width = 970 mm
Dead Load =	3.00 psf	Dead Load = 1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

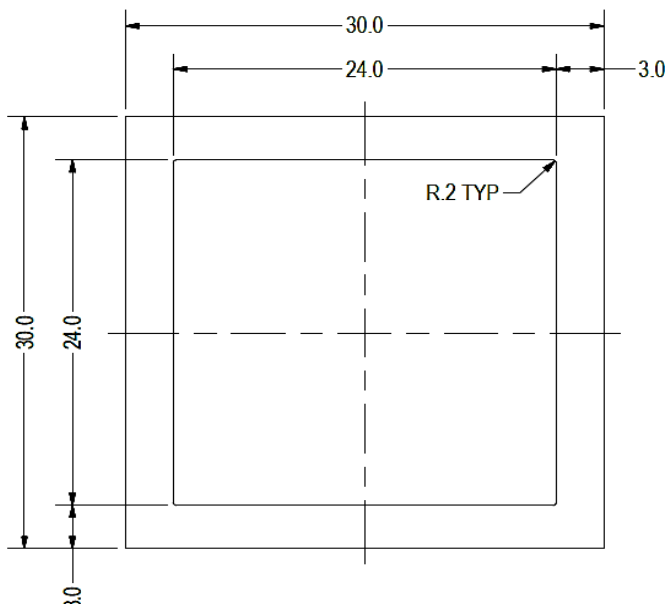
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

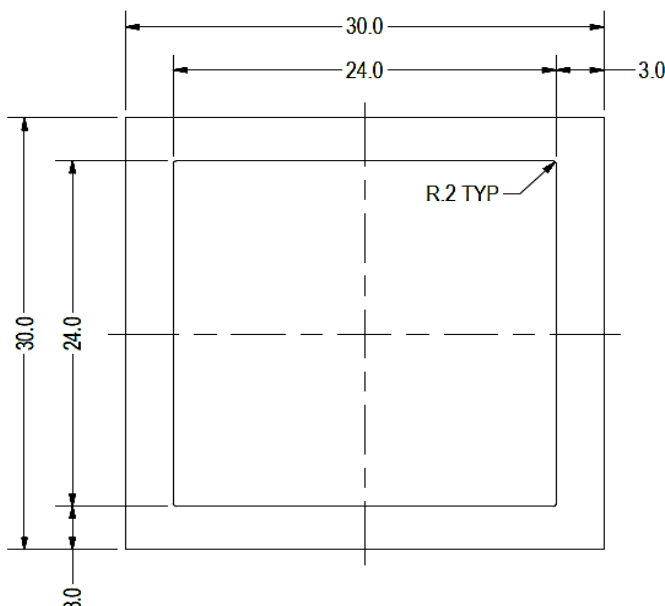
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.555 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	5%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.660 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	17%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.533 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.001 k-ft
P_n =	0.170 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	4%



A cross brace kit is required every 55 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

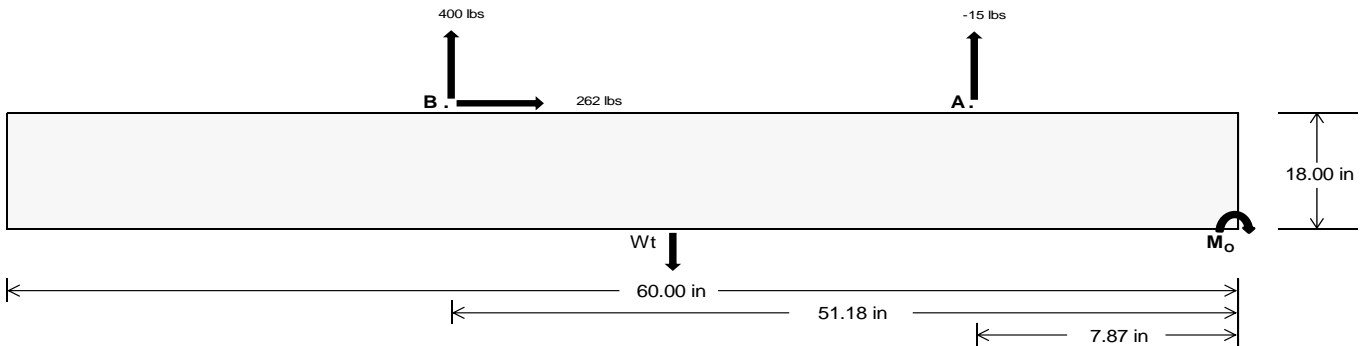
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>15.68</u>	<u>1665.73</u>	k
Compressive Load =	<u>721.74</u>	<u>1050.92</u>	k
Lateral Load =	<u>1.31</u>	<u>1090.99</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 25081.1$ in-lbs
Resisting Force Required = 836.04 lbs
S.F. = 1.67
Weight Required = 1393.39 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 262.22 lbs
Friction = 0.4
Weight Required = 655.55 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 262.22 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	223 lbs	223 lbs	223 lbs	223 lbs	304 lbs	304 lbs	304 lbs	304 lbs	372 lbs	372 lbs	372 lbs	372 lbs	30 lbs	30 lbs	30 lbs	30 lbs
F_B	137 lbs	137 lbs	137 lbs	137 lbs	480 lbs	480 lbs	480 lbs	480 lbs	447 lbs	447 lbs	447 lbs	447 lbs	-800 lbs	-800 lbs	-800 lbs	-800 lbs
F_V	15 lbs	15 lbs	15 lbs	15 lbs	468 lbs	468 lbs	468 lbs	468 lbs	361 lbs	361 lbs	361 lbs	361 lbs	-524 lbs	-524 lbs	-524 lbs	-524 lbs
P_{total}	2172 lbs	2263 lbs	2353 lbs	2444 lbs	2596 lbs	2687 lbs	2777 lbs	2868 lbs	2631 lbs	2722 lbs	2812 lbs	2903 lbs	317 lbs	372 lbs	426 lbs	480 lbs
M	192 lbs-ft	192 lbs-ft	192 lbs-ft	192 lbs-ft	416 lbs-ft	416 lbs-ft	416 lbs-ft	416 lbs-ft	438 lbs-ft	438 lbs-ft	438 lbs-ft	438 lbs-ft	681 lbs-ft	681 lbs-ft	681 lbs-ft	681 lbs-ft
e	0.09 ft	0.08 ft	0.08 ft	0.08 ft	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	2.15 ft	1.83 ft	1.60 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	233.0 psf	232.2 psf	231.6 psf	231.0 psf	251.6 psf	250.0 psf	248.5 psf	247.1 psf	252.6 psf	250.9 psf	249.4 psf	248.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	288.3 psf	284.9 psf	281.9 psf	279.1 psf	371.5 psf	364.2 psf	357.5 psf	351.4 psf	378.8 psf	371.1 psf	364.2 psf	357.8 psf	360.0 psf	212.4 psf	172.0 psf	154.5 psf

Maximum Bearing Pressure = 379 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

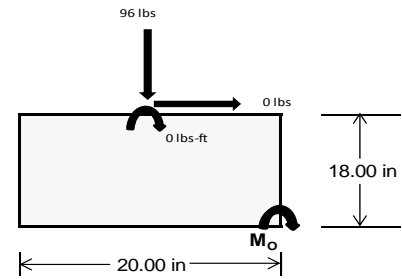
Overturning Check

$M_o = 0.0 \text{ ft-lbs}$
 Resisting Force Required = 0.00 lbs
 S.F. = 1.67
 Weight Required = 0.00 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	44 lbs	96 lbs	41 lbs	117 lbs	303 lbs	115 lbs	13 lbs	28 lbs	12 lbs
F_v	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2288 lbs	1813 lbs	2285 lbs	2253 lbs	2439 lbs	2251 lbs	669 lbs	1813 lbs	668 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
f_{min}	274.5 sqft	217.5 sqft	274.2 sqft	270.2 sqft	292.7 sqft	269.9 sqft	80.3 sqft	217.5 sqft	80.2 sqft
f_{max}	274.5 psf	217.5 psf	274.2 psf	270.5 psf	292.7 psf	270.2 psf	80.3 psf	217.5 psf	80.2 psf



Maximum Bearing Pressure = 293 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

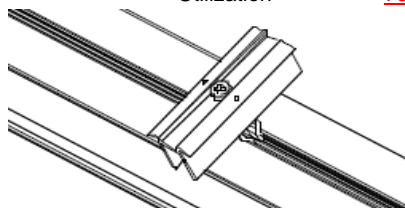
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

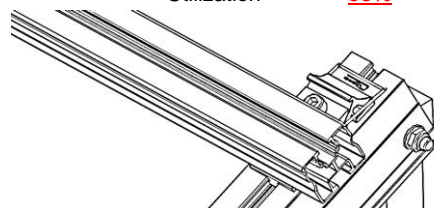
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.964 k
Allowable Uplift =	1.214 k
Utilization =	<u>79%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.039 k
Allowable Uplift =	1.116 k
Utilization =	<u>93%</u>



6.2 Bolted Connections

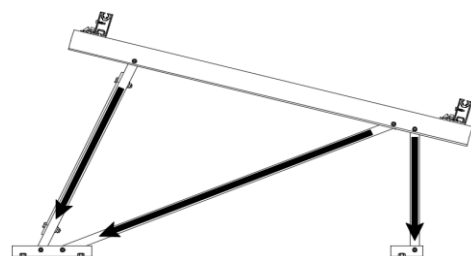
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.555 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>

Diagonal Strut

Maximum Axial Load =	0.660 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>



Rear Strut

Maximum Axial Load =	0.962 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Bracing

Maximum Axial Load =	0.170 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

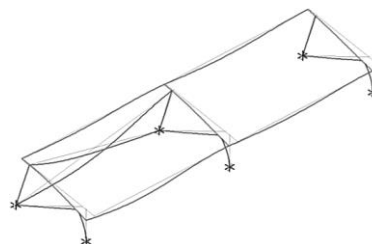
7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.002 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 36.00 \text{ in}$$

$$J = 0.255$$

$$93.7419$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.00 \text{ in}$$

$$J = 0.255$$

$$97.3454$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.3 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.287 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y F_{cy} \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	275.225	2	287.553	2	.008	10	0	10	0	1	0	1
2		min	-313.183	3	-437.778	3	-.165	3	0	3	0	1	0	1
3	N7	max	.027	3	191.767	1	.107	10	0	10	0	1	0	1
4		min	-.108	2	7.798	15	-.58	3	0	3	0	1	0	1
5	N15	max	.087	3	555.181	2	.025	9	0	9	0	1	0	1
6		min	-1.01	2	16.949	15	-.871	3	-.001	3	0	1	0	1
7	N16	max	754.632	2	808.401	2	0	15	0	9	0	1	0	1
8		min	-839.224	3	-1281.327	3	-108.235	3	0	3	0	1	0	1
9	N23	max	.028	3	192.154	1	.399	3	0	3	0	1	0	1
10		min	-.108	2	7.914	15	-.106	10	0	10	0	1	0	1
11	N24	max	275.225	2	289.671	2	109.453	3	0	9	0	1	0	1
12		min	-314.572	3	-438.382	3	-.008	10	0	3	0	1	0	1
13	Totals:	max	1303.856	2	2279.166	2	0	3						
14		min	-1466.838	3	-2005.474	3	0	2						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	182.144	2	.679	4	.035	9	0	10	0	10	0	1
2			min	-356.532	3	.16	15	-.066	3	0	3	0	3	0	1
3		2	max	182.279	2	.621	4	.035	9	0	10	0	10	0	15
4			min	-356.431	3	.146	15	-.066	3	0	3	0	3	0	4
5		3	max	182.414	2	.564	4	.035	9	0	10	0	10	0	15
6			min	-356.33	3	.133	15	-.066	3	0	3	0	3	0	4
7		4	max	182.549	2	.506	4	.035	9	0	10	0	10	0	15
8			min	-356.229	3	.119	15	-.066	3	0	3	0	3	0	4
9		5	max	182.684	2	.449	4	.035	9	0	10	0	10	0	15
10			min	-356.128	3	.106	15	-.066	3	0	3	0	3	0	4
11		6	max	182.819	2	.392	4	.035	9	0	10	0	9	0	15
12			min	-356.026	3	.092	15	-.066	3	0	3	0	3	0	4
13		7	max	182.954	2	.334	4	.035	9	0	10	0	9	0	15
14			min	-355.925	3	.079	15	-.066	3	0	3	0	3	0	4
15		8	max	183.089	2	.277	4	.035	9	0	10	0	9	0	15
16			min	-355.824	3	.065	15	-.066	3	0	3	0	3	0	4
17		9	max	183.223	2	.219	4	.035	9	0	10	0	9	0	15
18			min	-355.723	3	.052	15	-.066	3	0	3	0	3	0	4
19		10	max	183.358	2	.162	4	.035	9	0	10	0	9	0	15
20			min	-355.622	3	.037	12	-.066	3	0	3	0	3	0	4
21		11	max	183.493	2	.112	2	.035	9	0	10	0	9	0	15
22			min	-355.521	3	.015	12	-.066	3	0	3	0	3	0	4
23		12	max	183.628	2	.067	2	.035	9	0	10	0	9	0	15
24			min	-355.42	3	-.014	3	-.066	3	0	3	0	3	0	4
25		13	max	183.763	2	.022	2	.035	9	0	10	0	9	0	15
26			min	-355.318	3	-.047	3	-.066	3	0	3	0	3	0	4
27		14	max	183.898	2	-.016	15	.035	9	0	10	0	9	0	15
28			min	-355.217	3	-.081	3	-.066	3	0	3	0	3	0	4
29		15	max	184.033	2	-.03	15	.035	9	0	10	0	9	0	15
30			min	-355.116	3	-.126	4	-.066	3	0	3	0	3	0	4
31		16	max	184.167	2	-.043	15	.035	9	0	10	0	9	0	15
32			min	-355.015	3	-.183	4	-.066	3	0	3	0	3	0	4
33		17	max	184.302	2	-.057	15	.035	9	0	10	0	9	0	15
34			min	-354.914	3	-.241	4	-.066	3	0	3	0	3	0	4
35		18	max	184.437	2	-.07	15	.035	9	0	10	0	9	0	15
36			min	-354.813	3	-.298	4	-.066	3	0	3	0	3	0	4
37		19	max	184.572	2	-.084	15	.035	9	0	10	0	9	0	12



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-354.711	3	-.356	4	-.066	3	0	3	0	3	0	4
39	M3	1	max	244.621	2	1.736	4	.02	10	0	10	0	3	4
40		min	-225.109	3	.408	15	-.063	3	0	3	0	10	0	12
41		2	max	244.551	2	1.56	4	.02	10	0	10	0	3	2
42		min	-225.162	3	.367	15	-.063	3	0	3	0	10	0	3
43		3	max	244.481	2	1.383	4	.02	10	0	10	0	3	2
44		min	-225.214	3	.325	15	-.063	3	0	3	0	10	0	3
45		4	max	244.411	2	1.207	4	.02	10	0	10	0	3	15
46		min	-225.267	3	.284	15	-.063	3	0	3	0	10	0	4
47		5	max	244.341	2	1.031	4	.02	10	0	10	0	3	15
48		min	-225.319	3	.242	15	-.063	3	0	3	0	10	0	4
49		6	max	244.271	2	.854	4	.02	10	0	10	0	3	15
50		min	-225.372	3	.201	15	-.063	3	0	3	0	10	0	4
51		7	max	244.201	2	.678	4	.02	10	0	10	0	3	15
52		min	-225.424	3	.159	15	-.063	3	0	3	0	10	0	4
53		8	max	244.131	2	.502	4	.02	10	0	10	0	3	15
54		min	-225.477	3	.118	15	-.063	3	0	3	0	10	-.001	4
55		9	max	244.061	2	.325	4	.02	10	0	10	0	3	15
56		min	-225.529	3	.076	15	-.063	3	0	3	0	10	-.001	4
57		10	max	243.991	2	.149	4	.02	10	0	10	0	3	15
58		min	-225.582	3	.034	12	-.063	3	0	3	0	10	-.001	4
59		11	max	243.921	2	.007	2	.02	10	0	10	0	3	15
60		min	-225.634	3	-.054	3	-.063	3	0	3	0	10	-.001	4
61		12	max	243.851	2	-.048	15	.02	10	0	10	0	3	15
62		min	-225.687	3	-.204	4	-.063	3	0	3	0	10	-.001	4
63		13	max	243.781	2	-.089	15	.02	10	0	10	0	3	15
64		min	-225.739	3	-.38	4	-.063	3	0	3	0	10	-.001	4
65		14	max	243.711	2	-.131	15	.02	10	0	10	0	3	15
66		min	-225.792	3	-.557	4	-.063	3	0	3	0	10	-.001	4
67		15	max	243.641	2	-.172	15	.02	10	0	10	0	3	15
68		min	-225.844	3	-.733	4	-.063	3	0	3	0	10	0	4
69		16	max	243.571	2	-.214	15	.02	10	0	10	0	9	15
70		min	-225.897	3	-.909	4	-.063	3	0	3	0	10	0	4
71		17	max	243.501	2	-.255	15	.02	10	0	10	0	10	15
72		min	-225.949	3	-1.086	4	-.063	3	0	3	0	3	0	4
73		18	max	243.431	2	-.297	15	.02	10	0	10	0	10	15
74		min	-226.002	3	-1.262	4	-.063	3	0	3	0	3	0	4
75		19	max	243.361	2	-.338	15	.02	10	0	10	0	10	1
76		min	-226.054	3	-1.438	4	-.063	3	0	3	0	3	0	1
77	M4	1	max	190.603	1	0	1	.108	10	0	1	0	3	1
78		min	7.446	15	0	1	-.585	3	0	1	0	2	0	1
79		2	max	190.667	1	0	1	.108	10	0	1	0	10	1
80		min	7.466	15	0	1	-.585	3	0	1	0	3	0	1
81		3	max	190.732	1	0	1	.108	10	0	1	0	10	1
82		min	7.485	15	0	1	-.585	3	0	1	0	3	0	1
83		4	max	190.797	1	0	1	.108	10	0	1	0	10	1
84		min	7.505	15	0	1	-.585	3	0	1	0	3	0	1
85		5	max	190.861	1	0	1	.108	10	0	1	0	10	1
86		min	7.524	15	0	1	-.585	3	0	1	0	3	0	1
87		6	max	190.926	1	0	1	.108	10	0	1	0	10	1
88		min	7.544	15	0	1	-.585	3	0	1	0	3	0	1
89		7	max	190.991	1	0	1	.108	10	0	1	0	10	1
90		min	7.563	15	0	1	-.585	3	0	1	0	3	0	1
91		8	max	191.056	1	0	1	.108	10	0	1	0	10	1
92		min	7.583	15	0	1	-.585	3	0	1	0	3	0	1
93		9	max	191.12	1	0	1	.108	10	0	1	0	10	1
94		min	7.602	15	0	1	-.585	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	191.185	1	0	1	.108	10	0	1	0	10	0	1
96		min	7.622	15	0	1	-.585	3	0	1	0	3	0	1
97	11	max	191.25	1	0	1	.108	10	0	1	0	10	0	1
98		min	7.641	15	0	1	-.585	3	0	1	0	3	0	1
99	12	max	191.314	1	0	1	.108	10	0	1	0	10	0	1
100		min	7.661	15	0	1	-.585	3	0	1	0	3	0	1
101	13	max	191.379	1	0	1	.108	10	0	1	0	10	0	1
102		min	7.68	15	0	1	-.585	3	0	1	0	3	0	1
103	14	max	191.444	1	0	1	.108	10	0	1	0	10	0	1
104		min	7.7	15	0	1	-.585	3	0	1	0	3	0	1
105	15	max	191.508	1	0	1	.108	10	0	1	0	10	0	1
106		min	7.72	15	0	1	-.585	3	0	1	0	3	0	1
107	16	max	191.573	1	0	1	.108	10	0	1	0	10	0	1
108		min	7.739	15	0	1	-.585	3	0	1	0	3	0	1
109	17	max	191.638	1	0	1	.108	10	0	1	0	10	0	1
110		min	7.759	15	0	1	-.585	3	0	1	0	3	0	1
111	18	max	191.703	1	0	1	.108	10	0	1	0	10	0	1
112		min	7.778	15	0	1	-.585	3	0	1	0	3	0	1
113	19	max	191.767	1	0	1	.108	10	0	1	0	10	0	1
114		min	7.798	15	0	1	-.585	3	0	1	0	3	0	1
115	M6	1	max	530.618	2	.678	.001	9	0	3	0	3	0	1
116		min	-961.677	3	.16	15	-.317	3	0	13	0	13	0	1
117	2	max	530.753	2	.621	4	.001	9	0	3	0	3	0	15
118		min	-961.575	3	.146	15	-.317	3	0	13	0	13	0	4
119	3	max	530.888	2	.564	4	.001	9	0	3	0	3	0	15
120		min	-961.474	3	.133	15	-.317	3	0	13	0	13	0	4
121	4	max	531.023	2	.506	4	.001	9	0	3	0	3	0	15
122		min	-961.373	3	.119	15	-.317	3	0	13	0	13	0	4
123	5	max	531.158	2	.449	4	.001	9	0	3	0	3	0	15
124		min	-961.272	3	.104	12	-.317	3	0	13	0	13	0	4
125	6	max	531.293	2	.391	4	.001	9	0	3	0	3	0	15
126		min	-961.171	3	.081	12	-.317	3	0	13	0	13	0	4
127	7	max	531.428	2	.342	2	.001	9	0	3	0	3	0	15
128		min	-961.07	3	.059	12	-.317	3	0	13	0	13	0	4
129	8	max	531.562	2	.297	2	.001	9	0	3	0	9	0	15
130		min	-960.969	3	.036	12	-.317	3	0	13	0	3	0	4
131	9	max	531.697	2	.253	2	.001	9	0	3	0	9	0	12
132		min	-960.867	3	.014	3	-.317	3	0	13	0	3	0	4
133	10	max	531.832	2	.208	2	.001	9	0	3	0	9	0	12
134		min	-960.766	3	-.02	3	-.317	3	0	13	0	3	0	4
135	11	max	531.967	2	.163	2	.001	9	0	3	0	9	0	12
136		min	-960.665	3	-.053	3	-.317	3	0	13	0	3	0	4
137	12	max	532.102	2	.118	2	.001	9	0	3	0	9	0	12
138		min	-960.564	3	-.087	3	-.317	3	0	13	0	3	0	2
139	13	max	532.237	2	.073	2	.001	9	0	3	0	9	0	12
140		min	-960.463	3	-.12	3	-.317	3	0	13	0	3	0	2
141	14	max	532.372	2	.029	2	.001	9	0	3	0	9	0	12
142		min	-960.362	3	-.154	3	-.317	3	0	13	0	3	0	2
143	15	max	532.507	2	-.016	2	.001	9	0	3	0	9	0	12
144		min	-960.26	3	-.188	3	-.317	3	0	13	0	3	0	2
145	16	max	532.641	2	-.043	15	.001	9	0	3	0	9	0	12
146		min	-960.159	3	-.221	3	-.317	3	0	13	0	3	0	2
147	17	max	532.776	2	-.056	15	.001	9	0	3	0	9	0	3
148		min	-960.058	3	-.255	3	-.317	3	0	13	0	3	0	2
149	18	max	532.911	2	-.07	15	.001	9	0	3	0	9	0	3
150		min	-959.957	3	-.299	4	-.317	3	0	13	0	3	0	2
151	19	max	533.046	2	-.083	15	.001	9	0	3	0	9	0	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-959.856	3	-.356	4	-.317	3	0	13	0	3	0	2
153		max	659.86	2	1.738	4	.058	3	0	9	0	9	0	2
154		min	-555.715	3	.408	15	-.002	9	0	3	0	3	0	3
155		max	659.79	2	1.562	4	.058	3	0	9	0	9	0	2
156		min	-555.767	3	.367	15	-.002	9	0	3	0	3	0	3
157		max	659.72	2	1.386	4	.058	3	0	9	0	9	0	2
158		min	-555.82	3	.325	15	-.002	9	0	3	0	3	0	3
159		max	659.65	2	1.209	4	.058	3	0	9	0	9	0	2
160		min	-555.872	3	.284	15	-.002	9	0	3	0	3	0	3
161		max	659.58	2	1.033	4	.058	3	0	9	0	9	0	15
162		min	-555.925	3	.242	15	-.002	9	0	3	0	3	0	3
163		max	659.51	2	.857	4	.058	3	0	9	0	9	0	15
164		min	-555.977	3	.201	15	-.002	9	0	3	0	3	0	4
165		max	659.44	2	.68	4	.058	3	0	9	0	9	0	15
166		min	-556.03	3	.16	15	-.002	9	0	3	0	3	0	4
167		max	659.37	2	.504	4	.058	3	0	9	0	9	0	15
168		min	-556.082	3	.118	15	-.002	9	0	3	0	3	-.001	4
169		max	659.3	2	.329	2	.058	3	0	9	0	9	0	15
170	M8	min	-556.135	3	.071	12	-.002	9	0	3	0	3	-.001	4
171		max	659.23	2	.191	2	.058	3	0	9	0	9	0	15
172		min	-556.187	3	-.002	3	-.002	9	0	3	0	3	-.001	4
173		max	659.16	2	.054	2	.058	3	0	9	0	9	0	15
174		min	-556.24	3	-.105	3	-.002	9	0	3	0	3	-.001	4
175		max	659.09	2	-.048	15	.058	3	0	9	0	9	0	15
176		min	-556.292	3	-.208	3	-.002	9	0	3	0	3	-.001	4
177		max	659.02	2	-.089	15	.058	3	0	9	0	9	0	15
178		min	-556.345	3	-.378	4	-.002	9	0	3	0	3	-.001	4
179		max	658.95	2	-.131	15	.058	3	0	9	0	9	0	15
180		min	-556.397	3	-.554	4	-.002	9	0	3	0	3	-.001	4
181		max	658.88	2	-.172	15	.058	3	0	9	0	9	0	15
182		min	-556.45	3	-.731	4	-.002	9	0	3	0	3	0	4
183		max	658.81	2	-.214	15	.058	3	0	9	0	9	0	15
184		min	-556.502	3	-.907	4	-.002	9	0	3	0	3	0	4
185		max	658.74	2	-.255	15	.058	3	0	9	0	9	0	15
186		min	-556.555	3	-1.084	4	-.002	9	0	3	0	3	0	4
187		max	658.67	2	-.296	15	.058	3	0	9	0	9	0	15
188		min	-556.607	3	-1.26	4	-.002	9	0	3	0	3	0	4
189	M8	max	658.6	2	-.338	15	.058	3	0	9	0	9	0	1
190		min	-556.66	3	-1.436	4	-.002	9	0	3	0	3	0	1
191		max	554.016	2	0	1	.026	9	0	1	0	1	0	1
192		min	16.598	15	0	1	-.879	3	0	1	0	3	0	1
193		max	554.081	2	0	1	.026	9	0	1	0	9	0	1
194		min	16.617	15	0	1	-.879	3	0	1	0	3	0	1
195		max	554.145	2	0	1	.026	9	0	1	0	9	0	1
196		min	16.637	15	0	1	-.879	3	0	1	0	3	0	1
197		max	554.21	2	0	1	.026	9	0	1	0	9	0	1
198		min	16.656	15	0	1	-.879	3	0	1	0	3	0	1
199		max	554.275	2	0	1	.026	9	0	1	0	9	0	1
200		min	16.676	15	0	1	-.879	3	0	1	0	3	0	1
201		max	554.34	2	0	1	.026	9	0	1	0	9	0	1
202		min	16.695	15	0	1	-.879	3	0	1	0	3	0	1
203		max	554.404	2	0	1	.026	9	0	1	0	9	0	1
204		min	16.715	15	0	1	-.879	3	0	1	0	3	0	1
205		max	554.469	2	0	1	.026	9	0	1	0	9	0	1
206		min	16.734	15	0	1	-.879	3	0	1	0	3	0	1
207		max	554.534	2	0	1	.026	9	0	1	0	9	0	1
208		min	16.754	15	0	1	-.879	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	554.598	2	0	1	.026	9	0	1	0	9	0	1
210		min	16.773	15	0	1	-.879	3	0	1	0	3	0	1
211	11	max	554.663	2	0	1	.026	9	0	1	0	9	0	1
212		min	16.793	15	0	1	-.879	3	0	1	0	3	0	1
213	12	max	554.728	2	0	1	.026	9	0	1	0	9	0	1
214		min	16.812	15	0	1	-.879	3	0	1	0	3	0	1
215	13	max	554.793	2	0	1	.026	9	0	1	0	9	0	1
216		min	16.832	15	0	1	-.879	3	0	1	0	3	0	1
217	14	max	554.857	2	0	1	.026	9	0	1	0	9	0	1
218		min	16.852	15	0	1	-.879	3	0	1	-.001	3	0	1
219	15	max	554.922	2	0	1	.026	9	0	1	0	9	0	1
220		min	16.871	15	0	1	-.879	3	0	1	-.001	3	0	1
221	16	max	554.987	2	0	1	.026	9	0	1	0	9	0	1
222		min	16.891	15	0	1	-.879	3	0	1	-.001	3	0	1
223	17	max	555.051	2	0	1	.026	9	0	1	0	9	0	1
224		min	16.91	15	0	1	-.879	3	0	1	-.001	3	0	1
225	18	max	555.116	2	0	1	.026	9	0	1	0	9	0	1
226		min	16.93	15	0	1	-.879	3	0	1	-.001	3	0	1
227	19	max	555.181	2	0	1	.026	9	0	1	0	9	0	1
228		min	16.949	15	0	1	-.879	3	0	1	-.001	3	0	1
229	M10	1	max	183.242	2	.679	.011	10	0	1	0	9	0	1
230		min	-228.435	3	.16	15	-.036	9	0	3	0	3	0	1
231	2	max	183.377	2	.621	4	.011	10	0	1	0	9	0	15
232		min	-228.334	3	.146	15	-.036	9	0	3	0	3	0	4
233	3	max	183.512	2	.564	4	.011	10	0	1	0	9	0	15
234		min	-228.233	3	.133	15	-.036	9	0	3	0	3	0	4
235	4	max	183.647	2	.506	4	.011	10	0	1	0	9	0	15
236		min	-228.132	3	.119	15	-.036	9	0	3	0	3	0	4
237	5	max	183.782	2	.449	4	.011	10	0	1	0	9	0	15
238		min	-228.031	3	.106	15	-.036	9	0	3	0	3	0	4
239	6	max	183.916	2	.392	4	.011	10	0	1	0	9	0	15
240		min	-227.93	3	.092	15	-.036	9	0	3	0	3	0	4
241	7	max	184.051	2	.334	4	.011	10	0	1	0	9	0	15
242		min	-227.829	3	.079	15	-.036	9	0	3	0	3	0	4
243	8	max	184.186	2	.277	4	.011	10	0	1	0	10	0	15
244		min	-227.727	3	.065	15	-.036	9	0	3	0	3	0	4
245	9	max	184.321	2	.219	4	.011	10	0	1	0	10	0	15
246		min	-227.626	3	.052	15	-.036	9	0	3	0	3	0	4
247	10	max	184.456	2	.162	4	.011	10	0	1	0	10	0	15
248		min	-227.525	3	.038	12	-.036	9	0	3	0	3	0	4
249	11	max	184.591	2	.112	2	.011	10	0	1	0	10	0	15
250		min	-227.424	3	.015	12	-.036	9	0	3	0	3	0	4
251	12	max	184.726	2	.067	2	.011	10	0	1	0	10	0	15
252		min	-227.323	3	-.013	3	-.036	9	0	3	0	3	0	4
253	13	max	184.86	2	.022	2	.011	10	0	1	0	10	0	15
254		min	-227.222	3	-.047	3	-.036	9	0	3	0	3	0	4
255	14	max	184.995	2	-.016	15	.011	10	0	1	0	10	0	15
256		min	-227.12	3	-.08	3	-.036	9	0	3	0	3	0	4
257	15	max	185.13	2	-.03	15	.011	10	0	1	0	10	0	15
258		min	-227.019	3	-.126	4	-.036	9	0	3	0	3	0	4
259	16	max	185.265	2	-.043	15	.011	10	0	1	0	10	0	15
260		min	-226.918	3	-.183	4	-.036	9	0	3	0	3	0	4
261	17	max	185.4	2	-.057	15	.011	10	0	1	0	10	0	15
262		min	-226.817	3	-.241	4	-.036	9	0	3	0	3	0	4
263	18	max	185.535	2	-.07	15	.011	10	0	1	0	10	0	15
264		min	-226.716	3	-.298	4	-.036	9	0	3	0	3	0	4
265	19	max	185.67	2	-.084	15	.011	10	0	1	0	10	0	12



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-226.615	3	-.356	4	-.036	9	0	3	0	3	0	4
267	M11	1	max	244.233	2	1.736	4	.054	1	0	3	0	3	0	4
268			min	-226.437	3	.408	15	-.071	3	0	10	0	1	0	15
269		2	max	244.163	2	1.56	4	.054	1	0	3	0	3	0	2
270			min	-226.49	3	.367	15	-.071	3	0	10	0	1	0	12
271		3	max	244.093	2	1.383	4	.054	1	0	3	0	3	0	2
272			min	-226.542	3	.325	15	-.071	3	0	10	0	1	0	3
273		4	max	244.023	2	1.207	4	.054	1	0	3	0	3	0	15
274			min	-226.595	3	.284	15	-.071	3	0	10	0	1	0	4
275		5	max	243.953	2	1.031	4	.054	1	0	3	0	3	0	15
276			min	-226.647	3	.242	15	-.071	3	0	10	0	1	0	4
277		6	max	243.883	2	.854	4	.054	1	0	3	0	3	0	15
278			min	-226.7	3	.201	15	-.071	3	0	10	0	1	0	4
279		7	max	243.813	2	.678	4	.054	1	0	3	0	3	0	15
280			min	-226.752	3	.159	15	-.071	3	0	10	0	1	0	4
281		8	max	243.743	2	.502	4	.054	1	0	3	0	3	0	15
282			min	-226.805	3	.118	15	-.071	3	0	10	0	1	-.001	4
283		9	max	243.673	2	.325	4	.054	1	0	3	0	3	0	15
284			min	-226.857	3	.076	15	-.071	3	0	10	0	1	-.001	4
285		10	max	243.603	2	.149	4	.054	1	0	3	0	3	0	15
286			min	-226.91	3	.035	15	-.071	3	0	10	0	1	-.001	4
287		11	max	243.533	2	.007	2	.054	1	0	3	0	3	0	15
288			min	-226.962	3	-.044	3	-.071	3	0	10	0	1	-.001	4
289		12	max	243.463	2	-.048	15	.054	1	0	3	0	3	0	15
290			min	-227.015	3	-.204	4	-.071	3	0	10	0	1	-.001	4
291		13	max	243.393	2	-.089	15	.054	1	0	3	0	3	0	15
292			min	-227.067	3	-.38	4	-.071	3	0	10	0	1	-.001	4
293		14	max	243.323	2	-.131	15	.054	1	0	3	0	3	0	15
294			min	-227.12	3	-.557	4	-.071	3	0	10	0	1	-.001	4
295		15	max	243.253	2	-.172	15	.054	1	0	3	0	3	0	15
296			min	-227.172	3	-.733	4	-.071	3	0	10	0	1	0	4
297		16	max	243.183	2	-.214	15	.054	1	0	3	0	3	0	15
298			min	-227.225	3	-.909	4	-.071	3	0	10	0	1	0	4
299		17	max	243.113	2	-.255	15	.054	1	0	3	0	3	0	15
300			min	-227.277	3	-1.086	4	-.071	3	0	10	0	10	0	4
301		18	max	243.043	2	-.297	15	.054	1	0	3	0	3	0	15
302			min	-227.33	3	-1.262	4	-.071	3	0	10	0	10	0	4
303		19	max	242.973	2	-.338	15	.054	1	0	3	0	3	0	1
304			min	-227.382	3	-1.438	4	-.071	3	0	10	0	10	0	1
305	M12	1	max	190.989	1	0	1	.402	3	0	1	0	2	0	1
306			min	7.563	15	0	1	-.108	10	0	1	0	3	0	1
307		2	max	191.054	1	0	1	.402	3	0	1	0	1	0	1
308			min	7.582	15	0	1	-.108	10	0	1	0	10	0	1
309		3	max	191.118	1	0	1	.402	3	0	1	0	3	0	1
310			min	7.602	15	0	1	-.108	10	0	1	0	10	0	1
311		4	max	191.183	1	0	1	.402	3	0	1	0	3	0	1
312			min	7.621	15	0	1	-.108	10	0	1	0	10	0	1
313		5	max	191.248	1	0	1	.402	3	0	1	0	3	0	1
314			min	7.641	15	0	1	-.108	10	0	1	0	10	0	1
315		6	max	191.312	1	0	1	.402	3	0	1	0	3	0	1
316			min	7.66	15	0	1	-.108	10	0	1	0	10	0	1
317		7	max	191.377	1	0	1	.402	3	0	1	0	3	0	1
318			min	7.68	15	0	1	-.108	10	0	1	0	10	0	1
319		8	max	191.442	1	0	1	.402	3	0	1	0	3	0	1
320			min	7.699	15	0	1	-.108	10	0	1	0	10	0	1
321		9	max	191.507	1	0	1	.402	3	0	1	0	3	0	1
322			min	7.719	15	0	1	-.108	10	0	1	0	10	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-54.058	1	-171.572	3	-7.517	1	0	2	-.014	1	0	3
381	M5	1	max	152.478	1	1010.319	3	0	1	0	.016	3	0	3
382		min	-17.086	3	-598.261	2	-98.728	3	0	3	0	15	0	2
383		2	max	152.638	1	1010.148	3	0	1	0	0	9	.129	2
384		min	-16.966	3	-598.49	2	-98.728	3	0	3	-.005	3	-.218	3
385		3	max	295.493	3	4.136	9	10.322	3	0	0	9	.257	2
386		min	-72.172	2	-90.484	2	-.029	9	0	1	-.026	3	-.433	3
387		4	max	295.613	3	3.946	9	10.322	3	0	0	9	.277	2
388		min	-72.012	2	-90.713	2	-.029	9	0	1	-.023	3	-.425	3
389		5	max	295.733	3	3.755	9	10.322	3	0	0	9	.296	2
390		min	-71.852	2	-90.941	2	-.029	9	0	1	-.021	3	-.417	3
391		6	max	295.854	3	3.564	9	10.322	3	0	0	9	.316	2
392		min	-71.692	2	-91.17	2	-.029	9	0	1	-.019	3	-.409	3
393		7	max	295.974	3	3.374	9	10.322	3	0	0	9	.336	2
394		min	-71.532	2	-91.399	2	-.029	9	0	1	-.017	3	-.401	3
395		8	max	296.094	3	3.183	9	10.322	3	0	0	9	.356	2
396		min	-71.371	2	-91.628	2	-.029	9	0	1	-.014	3	-.394	3
397		9	max	296.214	3	2.993	9	10.322	3	0	0	9	.376	2
398		min	-71.211	2	-91.856	2	-.029	9	0	1	-.012	3	-.386	3
399		10	max	296.334	3	2.802	9	10.322	3	0	0	13	.396	2
400		min	-71.051	2	-92.085	2	-.029	9	0	1	-.01	3	-.378	3
401		11	max	296.454	3	2.611	9	10.322	3	0	0	1	.416	2
402		min	-70.891	2	-92.314	2	-.029	9	0	1	-.008	3	-.37	3
403		12	max	296.574	3	2.421	9	10.322	3	0	0	1	.436	2
404		min	-70.731	2	-92.543	2	-.029	9	0	1	-.006	3	-.362	3
405		13	max	296.694	3	2.23	9	10.322	3	0	0	1	.456	2
406		min	-70.571	2	-92.771	2	-.029	9	0	1	-.003	3	-.354	3
407		14	max	296.815	3	2.039	9	10.322	3	0	0	1	.476	2
408		min	-70.41	2	-93	2	-.029	9	0	1	-.001	3	-.345	3
409		15	max	296.935	3	1.849	9	10.322	3	0	.001	3	.496	2
410		min	-70.25	2	-93.229	2	-.029	9	0	1	0	9	-.337	3
411		16	max	239.779	2	490.118	2	10.311	3	0	.003	3	.511	2
412		min	2.919	15	-530.116	3	-.031	9	0	1	0	9	-.325	3
413		17	max	239.939	2	489.889	2	10.311	3	0	.005	3	.405	2
414		min	2.967	15	-530.287	3	-.031	9	0	1	0	9	-.21	3
415		18	max	4.668	3	939.481	2	9.428	3	0	.007	3	.203	2
416		min	-152.668	1	-484.673	3	-.006	9	0	9	0	9	-.105	3
417		19	max	4.788	3	939.252	2	9.428	3	0	.009	3	0	3
418		min	-152.508	1	-484.845	3	-.006	9	0	9	0	9	0	2
419	M9	1	max	54.073	1	334.965	3	105.289	3	0	.005	10	0	2
420		min	1.4	10	-204.807	2	-2.723	10	0	2	-.028	3	0	3
421		2	max	54.233	1	334.793	3	105.289	3	0	.005	10	.045	2
422		min	1.534	10	-205.036	2	-2.723	10	0	2	-.013	1	-.073	3
423		3	max	119.288	3	3.263	9	7.198	1	0	.017	3	.089	2
424		min	-35.977	2	-30.509	2	-2.712	10	0	10	-.011	1	-.144	3
425		4	max	119.408	3	3.073	9	7.198	1	0	.016	3	.095	2
426		min	-35.816	2	-30.738	2	-2.712	10	0	10	-.009	1	-.143	3
427		5	max	119.528	3	2.882	9	7.198	1	0	.015	3	.102	2
428		min	-35.656	2	-30.966	2	-2.712	10	0	10	-.008	1	-.141	3
429		6	max	119.648	3	2.692	9	7.198	1	0	.015	3	.109	2
430		min	-35.496	2	-31.195	2	-2.712	10	0	10	-.006	1	-.14	3
431		7	max	119.768	3	2.501	9	7.198	1	0	.014	3	.115	2
432		min	-35.336	2	-31.424	2	-2.712	10	0	10	-.005	1	-.138	3
433		8	max	119.889	3	2.31	9	7.198	1	0	.014	3	.122	2
434		min	-35.176	2	-31.652	2	-2.712	10	0	10	-.003	1	-.136	3
435		9	max	120.009	3	2.12	9	7.198	1	0	.013	3	.129	2
436		min	-35.016	2	-31.881	2	-2.712	10	0	10	-.002	1	-.135	3





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-2.723	10	-204.757	2	1.401	10	0	2	-.005	10	0	3
495	M16	1	max	2.838	10	317.579	2	-1.424	10	0	3	.014	1	0	2
496			min	-7.508	1	-171.596	3	-54.06	1	0	2	-.005	10	0	3
497		2	max	2.838	10	230.059	2	.381	10	0	3	.005	3	.05	3
498			min	-7.508	1	-125.596	3	-39.581	1	0	2	-.008	2	-.091	2
499		3	max	2.838	10	142.539	2	2.187	10	0	3	.001	3	.084	3
500			min	-7.508	1	-79.596	3	-25.102	1	0	2	-.012	1	-.153	2
501		4	max	2.838	10	55.019	2	3.992	10	0	3	0	15	.103	3
502			min	-7.508	1	-33.596	3	-10.623	1	0	2	-.018	1	-.186	2
503		5	max	2.838	10	12.404	3	9.099	2	0	3	0	15	.106	3
504			min	-7.508	1	-32.501	2	-8.293	3	0	2	-.019	1	-.19	2
505		6	max	2.838	10	58.404	3	18.335	1	0	3	0	10	.094	3
506			min	-7.508	1	-120.021	2	-7.342	3	0	2	-.016	1	-.165	2
507		7	max	2.838	10	104.404	3	32.815	1	0	3	.003	10	.067	3
508			min	-7.508	1	-207.541	2	-6.391	3	0	2	-.01	3	-.11	2
509		8	max	2.838	10	150.404	3	47.294	1	0	3	.01	2	.025	3
510			min	-7.508	1	-295.061	2	-5.44	3	0	2	-.012	3	-.026	2
511		9	max	2.838	10	196.404	3	61.773	1	0	3	.025	1	.087	2
512			min	-7.508	1	-382.581	2	-4.488	3	0	2	-.014	3	-.033	3
513		10	max	2.838	10	242.404	3	76.252	1	0	15	.048	1	.229	2
514			min	-7.508	1	-470.101	2	-3.537	3	0	2	-.015	3	-.106	3
515		11	max	2.838	10	382.581	2	2.2	3	0	2	.025	1	.087	2
516			min	-7.508	1	-196.404	3	-61.773	1	0	3	-.005	3	-.033	3
517		12	max	2.838	10	295.061	2	3.151	3	0	2	.01	2	.025	3
518			min	-7.508	1	-150.404	3	-47.294	1	0	3	-.004	3	-.026	2
519		13	max	2.838	10	207.541	2	4.102	3	0	2	.003	10	.067	3
520			min	-7.508	1	-104.404	3	-32.814	1	0	3	-.007	1	-.11	2
521		14	max	2.838	10	120.021	2	5.053	3	0	2	0	10	.094	3
522			min	-7.508	1	-58.404	3	-18.335	1	0	3	-.016	1	-.165	2
523		15	max	2.838	10	32.501	2	6.004	3	0	2	0	3	.106	3
524			min	-7.508	1	-12.404	3	-9.099	2	0	3	-.019	1	-.19	2
525		16	max	2.838	10	33.596	3	10.623	1	0	2	.003	3	.103	3
526			min	-7.508	1	-55.019	2	-3.992	10	0	3	-.018	1	-.186	2
527		17	max	2.838	10	79.596	3	25.102	1	0	2	.005	3	.084	3
528			min	-7.508	1	-142.539	2	-2.187	10	0	3	-.012	1	-.153	2
529		18	max	2.838	10	125.596	3	39.581	1	0	2	.008	3	.05	3
530			min	-7.508	1	-230.059	2	-.381	10	0	3	-.008	2	-.091	2
531		19	max	2.838	10	171.596	3	54.06	1	0	2	.014	1	0	2
532			min	-7.508	1	-317.579	2	1.425	10	0	3	-.005	10	0	3
533	M15	1	max	0	1	.648	3	.184	3	0	1	0	1	0	1
534			min	-168.589	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.576	3	.184	3	0	1	0	1	0	1
536			min	-168.664	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.504	3	.184	3	0	1	0	1	0	1
538			min	-168.74	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.432	3	.184	3	0	1	0	1	0	1
540			min	-168.815	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.36	3	.184	3	0	1	0	1	0	1
542			min	-168.891	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.288	3	.184	3	0	1	0	1	0	1
544			min	-168.966	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.216	3	.184	3	0	1	0	3	0	1
546			min	-169.042	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.144	3	.184	3	0	1	0	3	0	1
548			min	-169.117	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.072	3	.184	3	0	1	0	3	0	1
550			min	-169.193	3	0	1	0	1	0	3	0	1	0	3



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.184	3	0	1	0	3	0	1
552		min	-169.268	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.184	3	0	1	0	3	0	1
554		min	-169.344	3	-.072	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.184	3	0	1	0	3	0	1
556		min	-169.42	3	-.144	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.184	3	0	1	0	3	0	1
558		min	-169.495	3	-.216	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.184	3	0	1	0	3	0	1
560		min	-169.571	3	-.288	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.184	3	0	1	0	3	0	1
562		min	-169.646	3	-.36	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.184	3	0	1	0	3	0	1
564		min	-169.722	3	-.432	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.184	3	0	1	0	3	0	1
566		min	-169.797	3	-.504	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.184	3	0	1	0	3	0	1
568		min	-169.873	3	-.576	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.184	3	0	1	0	3	0	1
570		min	-169.948	3	-.648	3	0	1	0	3	0	1	0	1
571	M16A 1	max	0	1	1.109	4	.009	9	0	3	0	3	0	1
572		min	-167.225	3	0	1	-.078	3	0	9	0	9	0	1
573	2	max	0	1	.986	4	.009	9	0	3	0	3	0	1
574		min	-167.149	3	0	1	-.078	3	0	9	0	9	0	4
575	3	max	0	1	.863	4	.009	9	0	3	0	3	0	1
576		min	-167.074	3	0	1	-.078	3	0	9	0	9	0	4
577	4	max	0	1	.739	4	.009	9	0	3	0	3	0	1
578		min	-166.998	3	0	1	-.078	3	0	9	0	9	0	4
579	5	max	0	1	.616	4	.009	9	0	3	0	3	0	1
580		min	-166.923	3	0	1	-.078	3	0	9	0	9	0	4
581	6	max	0	1	.493	4	.009	9	0	3	0	3	0	1
582		min	-166.847	3	0	1	-.078	3	0	9	0	9	-.001	4
583	7	max	0	1	.37	4	.009	9	0	3	0	3	0	1
584		min	-166.771	3	0	1	-.078	3	0	9	0	9	-.001	4
585	8	max	0	1	.246	4	.009	9	0	3	0	3	0	1
586		min	-166.696	3	0	1	-.078	3	0	9	0	9	-.001	4
587	9	max	0	1	.123	4	.009	9	0	3	0	3	0	1
588		min	-166.62	3	0	1	-.078	3	0	9	0	9	-.001	4
589	10	max	0	1	0	1	.009	9	0	3	0	3	0	1
590		min	-166.545	3	0	1	-.078	3	0	9	0	9	-.001	4
591	11	max	.113	4	0	1	.009	9	0	3	0	3	0	1
592		min	-166.469	3	-.123	4	-.078	3	0	9	0	9	-.001	4
593	12	max	.242	4	0	1	.009	9	0	3	0	3	0	1
594		min	-166.394	3	-.246	4	-.078	3	0	9	0	4	-.001	4
595	13	max	.371	4	0	1	.009	9	0	3	0	1	0	1
596		min	-166.318	3	-.37	4	-.078	3	0	9	0	3	-.001	4
597	14	max	.5	4	0	1	.009	9	0	3	0	1	0	1
598		min	-166.243	3	-.493	4	-.078	3	0	9	0	3	-.001	4
599	15	max	.63	4	0	1	.009	9	0	3	0	9	0	1
600		min	-166.167	3	-.616	4	-.078	3	0	9	0	3	0	4
601	16	max	.759	4	0	1	.009	9	0	3	0	9	0	1
602		min	-166.092	3	-.739	4	-.078	3	0	9	0	3	0	4
603	17	max	.888	4	0	1	.009	9	0	3	0	9	0	1
604		min	-166.016	3	-.863	4	-.078	3	0	9	0	3	0	4
605	18	max	1.017	4	0	1	.009	9	0	3	0	9	0	1
606		min	-165.941	3	-.986	4	-.078	3	0	9	0	3	0	4
607	19	max	1.146	4	0	1	.009	9	0	3	0	9	0	1



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Envelope Member Section Forces (Continued)

Member	Sec	Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC	
608		min	-165.865	3	-1.109	4	-0.078	3	0	9	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.011	2	0	9	4.552e-5	10	NC	3	NC	1
2			min	-.004	3	-.011	3	-.003	3	-2.53e-4	3	3924.883	2	NC	1
3		2	max	.002	2	.01	2	0	9	4.334e-5	10	NC	3	NC	1
4			min	-.004	3	-.011	3	-.003	3	-2.391e-4	3	4292.811	2	NC	1
5		3	max	.002	2	.009	2	0	9	4.116e-5	10	NC	3	NC	1
6			min	-.003	3	-.01	3	-.003	3	-2.252e-4	3	4732.079	2	NC	1
7		4	max	.002	2	.008	2	0	9	3.897e-5	10	NC	1	NC	1
8			min	-.003	3	-.01	3	-.002	3	-2.113e-4	3	5260.135	2	NC	1
9		5	max	.001	2	.007	2	0	9	3.679e-5	10	NC	1	NC	1
10			min	-.003	3	-.009	3	-.002	3	-1.974e-4	3	5900.294	2	NC	1
11		6	max	.001	2	.006	2	0	9	3.46e-5	10	NC	1	NC	1
12			min	-.003	3	-.009	3	-.002	3	-1.835e-4	3	6684.218	2	NC	1
13		7	max	.001	2	.006	2	0	9	3.242e-5	10	NC	1	NC	1
14			min	-.002	3	-.008	3	-.002	3	-1.696e-4	3	7655.721	2	NC	1
15		8	max	.001	2	.005	2	0	9	3.024e-5	10	NC	1	NC	1
16			min	-.002	3	-.008	3	-.001	3	-1.558e-4	3	8876.773	2	NC	1
17		9	max	.001	2	.004	2	0	9	2.805e-5	10	NC	1	NC	1
18			min	-.002	3	-.007	3	-.001	3	-1.419e-4	3	NC	1	NC	1
19		10	max	0	2	.003	2	0	9	2.587e-5	10	NC	1	NC	1
20			min	-.002	3	-.007	3	0	3	-1.28e-4	3	NC	1	NC	1
21		11	max	0	2	.003	2	0	9	2.369e-5	10	NC	1	NC	1
22			min	-.002	3	-.006	3	0	3	-1.141e-4	3	NC	1	NC	1
23		12	max	0	2	.002	2	0	9	2.15e-5	10	NC	1	NC	1
24			min	-.001	3	-.005	3	0	3	-1.002e-4	3	NC	1	NC	1
25		13	max	0	2	.002	2	0	9	1.932e-5	10	NC	1	NC	1
26			min	-.001	3	-.005	3	0	3	-8.631e-5	3	NC	1	NC	1
27		14	max	0	2	.001	2	0	9	1.713e-5	10	NC	1	NC	1
28			min	-.001	3	-.004	3	0	3	-7.242e-5	3	NC	1	NC	1
29		15	max	0	2	0	2	0	9	1.495e-5	10	NC	1	NC	1
30			min	0	3	-.003	3	0	3	-5.853e-5	3	NC	1	NC	1
31		16	max	0	2	0	2	0	9	1.277e-5	10	NC	1	NC	1
32			min	0	3	-.002	3	0	3	-4.464e-5	3	NC	1	NC	1
33		17	max	0	2	0	2	0	9	1.058e-5	10	NC	1	NC	1
34			min	0	3	-.002	3	0	3	-3.075e-5	3	NC	1	NC	1
35		18	max	0	2	0	2	0	9	8.4e-6	10	NC	1	NC	1
36			min	0	3	0	3	0	3	-2.311e-5	9	NC	1	NC	1
37		19	max	0	1	0	1	0	1	6.216e-6	10	NC	1	NC	1
38			min	0	1	0	1	0	1	-1.779e-5	9	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	8.548e-6	9	NC	1	NC	1
40			min	0	1	0	1	0	1	-2.994e-6	10	NC	1	NC	1
41		2	max	0	3	0	2	0	10	1.142e-5	9	NC	1	NC	1
42			min	0	2	0	3	0	9	-4.182e-6	10	NC	1	NC	1
43		3	max	0	3	0	2	0	3	1.437e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.369e-6	10	NC	1	NC	1
45		4	max	0	3	0	2	0	3	1.761e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	9	-6.557e-6	10	NC	1	NC	1
47		5	max	0	3	0	2	0	3	2.085e-5	1	NC	1	NC	1
48			min	0	2	-.004	3	0	9	-7.745e-6	10	NC	1	NC	1
49		6	max	0	3	0	2	0	3	2.408e-5	1	NC	1	NC	1
50			min	0	2	-.005	3	0	9	-8.933e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	2.732e-5	1	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	10	-1.012e-5	10	NC	1	NC	1
53	8	max	0	3	.001	2	0	3	3.056e-5	1	NC	1	NC	1
54		min	-.001	2	-.006	3	0	10	-1.131e-5	10	NC	1	NC	1
55	9	max	.001	3	.001	2	0	3	3.379e-5	1	NC	1	NC	1
56		min	-.001	2	-.007	3	0	10	-1.25e-5	10	NC	1	NC	1
57	10	max	.001	3	.002	2	0	3	3.703e-5	1	NC	1	NC	1
58		min	-.001	2	-.007	3	0	10	-1.368e-5	10	NC	1	NC	1
59	11	max	.001	3	.002	2	0	3	4.027e-5	1	NC	1	NC	1
60		min	-.002	2	-.008	3	0	10	-1.487e-5	10	NC	1	NC	1
61	12	max	.002	3	.003	2	0	3	4.35e-5	1	NC	1	NC	1
62		min	-.002	2	-.008	3	0	10	-1.606e-5	10	NC	1	NC	1
63	13	max	.002	3	.004	2	.001	3	4.724e-5	3	NC	1	NC	1
64		min	-.002	2	-.008	3	0	10	-1.725e-5	10	NC	1	NC	1
65	14	max	.002	3	.004	2	.001	3	5.103e-5	3	NC	1	NC	1
66		min	-.002	2	-.009	3	0	10	-1.844e-5	10	NC	1	NC	1
67	15	max	.002	3	.005	2	.001	3	5.482e-5	3	NC	1	NC	1
68		min	-.002	2	-.009	3	0	10	-1.962e-5	10	8557.174	2	NC	1
69	16	max	.002	3	.006	2	.002	3	5.861e-5	3	NC	1	NC	1
70		min	-.002	2	-.009	3	0	10	-2.081e-5	10	7264.34	2	NC	1
71	17	max	.002	3	.007	2	.002	3	6.24e-5	3	NC	1	NC	1
72		min	-.002	2	-.009	3	0	10	-2.2e-5	10	6261.651	2	NC	1
73	18	max	.002	3	.008	2	.002	3	6.619e-5	3	NC	1	NC	1
74		min	-.003	2	-.009	3	0	10	-2.319e-5	10	5475.275	2	NC	1
75	19	max	.003	3	.009	2	.002	3	6.998e-5	3	NC	3	NC	1
76		min	-.003	2	-.009	3	0	10	-2.437e-5	10	4853.133	2	NC	1
77	M4	1	max	0	.012	2	0	10	3.053e-5	10	NC	1	NC	1
78		min	0	15	-.011	3	-.002	3	-8.119e-5	1	NC	1	NC	1
79	2	max	0	1	.012	2	0	10	3.053e-5	10	NC	1	NC	1
80		min	0	15	-.01	3	-.002	3	-8.119e-5	1	NC	1	NC	1
81	3	max	0	1	.011	2	0	10	3.053e-5	10	NC	1	NC	1
82		min	0	15	-.01	3	-.002	3	-8.119e-5	1	NC	1	NC	1
83	4	max	0	1	.01	2	0	10	3.053e-5	10	NC	1	NC	1
84		min	0	15	-.009	3	-.001	3	-8.119e-5	1	NC	1	NC	1
85	5	max	0	1	.01	2	0	10	3.053e-5	10	NC	1	NC	1
86		min	0	15	-.009	3	-.001	3	-8.119e-5	1	NC	1	NC	1
87	6	max	0	1	.009	2	0	10	3.053e-5	10	NC	1	NC	1
88		min	0	15	-.008	3	-.001	3	-8.119e-5	1	NC	1	NC	1
89	7	max	0	1	.008	2	0	10	3.053e-5	10	NC	1	NC	1
90		min	0	15	-.007	3	0	3	-8.119e-5	1	NC	1	NC	1
91	8	max	0	1	.008	2	0	10	3.053e-5	10	NC	1	NC	1
92		min	0	15	-.007	3	0	3	-8.119e-5	1	NC	1	NC	1
93	9	max	0	1	.007	2	0	10	3.053e-5	10	NC	1	NC	1
94		min	0	15	-.006	3	0	3	-8.119e-5	1	NC	1	NC	1
95	10	max	0	1	.006	2	0	10	3.053e-5	10	NC	1	NC	1
96		min	0	15	-.006	3	0	3	-8.119e-5	1	NC	1	NC	1
97	11	max	0	1	.006	2	0	10	3.053e-5	10	NC	1	NC	1
98		min	0	15	-.005	3	0	3	-8.119e-5	1	NC	1	NC	1
99	12	max	0	1	.005	2	0	10	3.053e-5	10	NC	1	NC	1
100		min	0	15	-.004	3	0	3	-8.119e-5	1	NC	1	NC	1
101	13	max	0	1	.004	2	0	10	3.053e-5	10	NC	1	NC	1
102		min	0	15	-.004	3	0	3	-8.119e-5	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	10	3.053e-5	10	NC	1	NC	1
104		min	0	15	-.003	3	0	3	-8.119e-5	1	NC	1	NC	1
105	15	max	0	1	.003	2	0	10	3.053e-5	10	NC	1	NC	1
106		min	0	15	-.002	3	0	3	-8.119e-5	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	10	3.053e-5	10	NC	1	NC	1
108		min	0	15	-.002	3	0	3	-8.119e-5	1	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	3.053e-5	10	NC	1	NC	1
110			min	0	15	-.001	3	0	3	-8.119e-5	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	3.053e-5	10	NC	1	NC	1
112			min	0	15	0	3	0	3	-8.119e-5	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	3.053e-5	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-8.119e-5	1	NC	1	NC	1
115	M6	1	max	.006	2	.03	2	0	9	5.667e-4	3	NC	3	NC	1
116			min	-.01	3	-.03	3	-.008	3	-2.401e-7	9	1398.14	2	5303.046	3
117		2	max	.005	2	.028	2	0	9	5.488e-4	3	NC	3	NC	1
118			min	-.009	3	-.028	3	-.008	3	-5.066e-7	9	1499.896	2	5598.831	3
119		3	max	.005	2	.026	2	0	9	5.309e-4	3	NC	3	NC	1
120			min	-.009	3	-.027	3	-.007	3	-7.731e-7	9	1617.017	2	5955.54	3
121		4	max	.005	2	.024	2	0	9	5.13e-4	3	NC	3	NC	1
122			min	-.008	3	-.025	3	-.007	3	-1.04e-6	9	1752.592	2	6385.534	3
123		5	max	.004	2	.022	2	0	9	4.95e-4	3	NC	3	NC	1
124			min	-.008	3	-.024	3	-.006	3	-1.306e-6	9	1910.605	2	6905.382	3
125		6	max	.004	2	.02	2	0	9	4.771e-4	3	NC	3	NC	1
126			min	-.007	3	-.022	3	-.006	3	-1.572e-6	9	2096.276	2	7537.538	3
127		7	max	.004	2	.018	2	0	9	4.592e-4	3	NC	3	NC	1
128			min	-.007	3	-.021	3	-.005	3	-1.839e-6	9	2316.567	2	8312.913	3
129		8	max	.003	2	.016	2	0	9	4.412e-4	3	NC	3	NC	1
130			min	-.006	3	-.019	3	-.005	3	-2.105e-6	9	2580.976	2	9274.926	3
131		9	max	.003	2	.015	2	0	9	4.233e-4	3	NC	3	NC	1
132			min	-.006	3	-.017	3	-.004	3	-2.372e-6	9	2902.794	2	NC	1
133		10	max	.003	2	.013	2	0	9	4.054e-4	3	NC	3	NC	1
134			min	-.005	3	-.016	3	-.004	3	-2.638e-6	9	3301.214	2	NC	1
135		11	max	.002	2	.011	2	0	9	3.875e-4	3	NC	3	NC	1
136			min	-.004	3	-.014	3	-.003	3	-2.905e-6	9	3805.002	2	NC	1
137		12	max	.002	2	.01	2	0	9	3.695e-4	3	NC	3	NC	1
138			min	-.004	3	-.012	3	-.003	3	-3.171e-6	9	4459.33	2	NC	1
139		13	max	.002	2	.008	2	0	9	3.516e-4	3	NC	1	NC	1
140			min	-.003	3	-.011	3	-.002	3	-3.438e-6	9	5339.43	2	NC	1
141		14	max	.002	2	.006	2	0	9	3.337e-4	3	NC	1	NC	1
142			min	-.003	3	-.009	3	-.002	3	-3.704e-6	9	6580.625	2	NC	1
143		15	max	.001	2	.005	2	0	9	3.158e-4	3	NC	1	NC	1
144			min	-.002	3	-.007	3	-.001	3	-3.971e-6	9	8453.423	2	NC	1
145		16	max	0	2	.004	2	0	9	2.978e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-4.237e-6	9	NC	1	NC	1
147		17	max	0	2	.002	2	0	9	2.799e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-4.504e-6	9	NC	1	NC	1
149		18	max	0	2	.001	2	0	9	2.62e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-4.77e-6	9	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.441e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-5.036e-6	9	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.405e-6	9	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.162e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	2.201e-6	9	NC	1	NC	1
156			min	0	2	-.002	3	0	9	-8.783e-5	3	NC	1	NC	1
157		3	max	0	3	.002	2	.001	3	1.996e-6	9	NC	1	NC	1
158			min	0	2	-.004	3	0	9	-5.948e-5	3	NC	1	NC	1
159		4	max	.001	3	.003	2	.002	3	1.792e-6	9	NC	1	NC	1
160			min	-.001	2	-.006	3	0	9	-3.113e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	1.588e-6	9	NC	1	NC	1
162			min	-.002	2	-.007	3	0	9	-2.782e-6	3	NC	1	NC	1
163		6	max	.002	3	.006	2	.002	3	2.557e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	9	0	5	8056.551	2	NC	1
165		7	max	.002	3	.007	2	.003	3	5.392e-5	3	NC	1	NC	1



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Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.011	3	0	9	0	5	6666.518	2	NC	1
167		8	max	.002	3	.008	2	.003	3	8.227e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	0	5	5645.726	2	NC	1
169		9	max	.003	3	.009	2	.003	3	1.106e-4	3	NC	1	NC	1
170			min	-.003	2	-.014	3	0	9	0	5	4860.513	2	NC	1
171		10	max	.003	3	.011	2	.003	3	1.39e-4	3	NC	3	NC	1
172			min	-.004	2	-.015	3	0	9	0	5	4236.682	2	NC	1
173		11	max	.004	3	.012	2	.004	3	1.673e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	0	15	3729.533	2	NC	1
175		12	max	.004	3	.014	2	.004	3	1.957e-4	3	NC	3	NC	1
176			min	-.005	2	-.018	3	0	9	0	15	3310.313	2	NC	1
177		13	max	.004	3	.016	2	.004	3	2.24e-4	3	NC	3	NC	1
178			min	-.005	2	-.019	3	0	9	-4.634e-8	9	2959.496	2	NC	1
179		14	max	.005	3	.017	2	.004	3	2.524e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-2.506e-7	9	2663.215	2	NC	1
181		15	max	.005	3	.019	2	.004	3	2.807e-4	3	NC	3	NC	1
182			min	-.006	2	-.021	3	0	9	-4.549e-7	9	2411.255	2	NC	1
183		16	max	.005	3	.021	2	.004	3	3.091e-4	3	NC	3	NC	1
184			min	-.006	2	-.022	3	0	9	-6.591e-7	9	2195.862	2	NC	1
185		17	max	.006	3	.023	2	.004	3	3.374e-4	3	NC	3	NC	1
186			min	-.007	2	-.023	3	0	9	-8.634e-7	9	2011.016	2	NC	1
187		18	max	.006	3	.025	2	.004	3	3.658e-4	3	NC	3	NC	1
188			min	-.007	2	-.024	3	0	9	-1.068e-6	9	1851.958	2	NC	1
189		19	max	.006	3	.027	2	.004	3	3.941e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	9	-1.272e-6	9	1714.881	2	NC	1
191	M8	1	max	.003	2	.035	2	0	9	-5.27e-8	15	NC	1	NC	1
192			min	0	15	-.03	3	-.003	3	-2.674e-4	3	NC	1	6955.533	3
193		2	max	.002	2	.033	2	0	9	-5.27e-8	15	NC	1	NC	1
194			min	0	15	-.028	3	-.003	3	-2.674e-4	3	NC	1	7583.727	3
195		3	max	.002	2	.031	2	0	9	-5.27e-8	15	NC	1	NC	1
196			min	0	15	-.027	3	-.002	3	-2.674e-4	3	NC	1	8331.578	3
197		4	max	.002	2	.029	2	0	9	-5.27e-8	15	NC	1	NC	1
198			min	0	15	-.025	3	-.002	3	-2.674e-4	3	NC	1	9230.607	3
199		5	max	.002	2	.027	2	0	9	-5.27e-8	15	NC	1	NC	1
200			min	0	15	-.023	3	-.002	3	-2.674e-4	3	NC	1	NC	1
201		6	max	.002	2	.025	2	0	9	-5.27e-8	15	NC	1	NC	1
202			min	0	15	-.022	3	-.002	3	-2.674e-4	3	NC	1	NC	1
203		7	max	.002	2	.023	2	0	9	-5.27e-8	15	NC	1	NC	1
204			min	0	15	-.02	3	-.001	3	-2.674e-4	3	NC	1	NC	1
205		8	max	.002	2	.022	2	0	9	-5.27e-8	15	NC	1	NC	1
206			min	0	15	-.018	3	-.001	3	-2.674e-4	3	NC	1	NC	1
207		9	max	.001	2	.02	2	0	9	-5.27e-8	15	NC	1	NC	1
208			min	0	15	-.017	3	-.001	3	-2.674e-4	3	NC	1	NC	1
209		10	max	.001	2	.018	2	0	9	-5.27e-8	15	NC	1	NC	1
210			min	0	15	-.015	3	0	3	-2.674e-4	3	NC	1	NC	1
211		11	max	.001	2	.016	2	0	9	-5.27e-8	15	NC	1	NC	1
212			min	0	15	-.013	3	0	3	-2.674e-4	3	NC	1	NC	1
213		12	max	.001	2	.014	2	0	9	-5.27e-8	15	NC	1	NC	1
214			min	0	15	-.012	3	0	3	-2.674e-4	3	NC	1	NC	1
215		13	max	0	2	.012	2	0	9	-5.27e-8	15	NC	1	NC	1
216			min	0	15	-.01	3	0	3	-2.674e-4	3	NC	1	NC	1
217		14	max	0	2	.01	2	0	9	-5.27e-8	15	NC	1	NC	1
218			min	0	15	-.008	3	0	3	-2.674e-4	3	NC	1	NC	1
219		15	max	0	2	.008	2	0	9	-5.27e-8	15	NC	1	NC	1
220			min	0	15	-.007	3	0	3	-2.674e-4	3	NC	1	NC	1
221		16	max	0	2	.006	2	0	9	-5.27e-8	15	NC	1	NC	1
222			min	0	15	-.005	3	0	3	-2.674e-4	3	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	2	.004	2	0	9	-5.27e-8	15	NC	1	NC	1
224			min	0	15	-.003	3	0	3	-2.674e-4	3	NC	1	NC	1
225		18	max	0	2	.002	2	0	9	-5.27e-8	15	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.674e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-5.27e-8	15	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.674e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.011	2	0	10	1.191e-4	1	NC	3	NC	1
230			min	-.002	3	-.011	3	0	3	-6.375e-4	3	3927.767	2	NC	1
231		2	max	.002	2	.01	2	0	10	1.134e-4	1	NC	3	NC	1
232			min	-.002	3	-.011	3	0	1	-6.141e-4	3	4296.066	2	NC	1
233		3	max	.002	2	.009	2	0	10	1.077e-4	1	NC	3	NC	1
234			min	-.002	3	-.01	3	0	1	-5.908e-4	3	4735.801	2	NC	1
235		4	max	.002	2	.008	2	0	10	1.02e-4	1	NC	1	NC	1
236			min	-.002	3	-.01	3	0	1	-5.674e-4	3	5264.443	2	NC	1
237		5	max	.002	2	.007	2	0	10	9.632e-5	1	NC	1	NC	1
238			min	-.002	3	-.009	3	0	1	-5.44e-4	3	5905.35	2	NC	1
239		6	max	.001	2	.006	2	0	10	9.061e-5	1	NC	1	NC	1
240			min	-.002	3	-.009	3	0	1	-5.206e-4	3	6690.239	2	NC	1
241		7	max	.001	2	.006	2	0	3	8.49e-5	1	NC	1	NC	1
242			min	-.002	3	-.008	3	0	1	-4.972e-4	3	7663.006	2	NC	1
243		8	max	.001	2	.005	2	0	3	7.919e-5	1	NC	1	NC	1
244			min	-.001	3	-.008	3	0	1	-4.738e-4	3	8885.744	2	NC	1
245		9	max	.001	2	.004	2	0	3	7.349e-5	1	NC	1	NC	1
246			min	-.001	3	-.007	3	0	1	-4.504e-4	3	NC	1	NC	1
247		10	max	0	2	.003	2	0	3	6.778e-5	1	NC	1	NC	1
248			min	-.001	3	-.007	3	0	1	-4.27e-4	3	NC	1	NC	1
249		11	max	0	2	.003	2	0	3	6.207e-5	1	NC	1	NC	1
250			min	-.001	3	-.006	3	0	1	-4.036e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	5.636e-5	1	NC	1	NC	1
252			min	0	3	-.005	3	0	1	-3.802e-4	3	NC	1	NC	1
253		13	max	0	2	.002	2	0	3	5.066e-5	1	NC	1	NC	1
254			min	0	3	-.005	3	0	1	-3.568e-4	3	NC	1	NC	1
255		14	max	0	2	.001	2	0	3	4.495e-5	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-3.334e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	3.924e-5	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-3.1e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	3.353e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.866e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	2.783e-5	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-2.632e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	2.212e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.398e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.641e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.164e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.036e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-7.902e-6	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.74e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.112e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.116e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.434e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	1	2.492e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-1.757e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	1	7.793e-6	10	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-2.079e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	8.994e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-2.755e-5	3	NC	1	NC	1
279		7	max	0	3	0	2	0	10	1.019e-5	10	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280		min	0	2	-.005	3	-.002	3	-5.378e-5	3	NC	1	NC	1
281	8	max	.001	3	.001	2	0	10	1.139e-5	10	NC	1	NC	1
282		min	-.001	2	-.006	3	-.002	3	-8.002e-5	3	NC	1	NC	1
283	9	max	.001	3	.001	2	0	10	1.259e-5	10	NC	1	NC	1
284		min	-.001	2	-.007	3	-.003	3	-1.063e-4	3	NC	1	NC	1
285	10	max	.001	3	.002	2	0	10	1.379e-5	10	NC	1	NC	1
286		min	-.001	2	-.007	3	-.003	3	-1.325e-4	3	NC	1	NC	1
287	11	max	.001	3	.002	2	0	10	1.499e-5	10	NC	1	NC	1
288		min	-.002	2	-.008	3	-.003	3	-1.587e-4	3	NC	1	NC	1
289	12	max	.002	3	.003	2	0	10	1.619e-5	10	NC	1	NC	1
290		min	-.002	2	-.008	3	-.003	3	-1.85e-4	3	NC	1	NC	1
291	13	max	.002	3	.004	2	0	10	1.739e-5	10	NC	1	NC	1
292		min	-.002	2	-.008	3	-.003	3	-2.112e-4	3	NC	1	NC	1
293	14	max	.002	3	.004	2	0	10	1.859e-5	10	NC	1	NC	1
294		min	-.002	2	-.009	3	-.003	3	-2.374e-4	3	NC	1	NC	1
295	15	max	.002	3	.005	2	0	10	1.98e-5	10	NC	1	NC	1
296		min	-.002	2	-.009	3	-.003	3	-2.637e-4	3	8567.459	2	NC	1
297	16	max	.002	3	.006	2	0	10	2.1e-5	10	NC	1	NC	1
298		min	-.002	2	-.009	3	-.003	3	-2.899e-4	3	7272.275	2	NC	1
299	17	max	.002	3	.007	2	0	10	2.22e-5	10	NC	1	NC	1
300		min	-.002	2	-.009	3	-.003	3	-3.161e-4	3	6267.934	2	NC	1
301	18	max	.002	3	.008	2	0	10	2.34e-5	10	NC	1	NC	1
302		min	-.003	2	-.009	3	-.002	3	-3.424e-4	3	5480.376	2	NC	1
303	19	max	.003	3	.009	2	0	10	2.46e-5	10	NC	3	NC	1
304		min	-.003	2	-.009	3	-.002	3	-3.686e-4	3	4857.373	2	NC	1
305	M12	1	max	0	.012	2	.001	3	4.448e-4	3	NC	1	NC	1
306		min	0	15	-.011	3	0	10	-3.082e-5	10	NC	1	NC	1
307	2	max	0	1	.012	2	.001	3	4.448e-4	3	NC	1	NC	1
308		min	0	15	-.011	3	0	10	-3.082e-5	10	NC	1	NC	1
309	3	max	0	1	.011	2	.001	3	4.448e-4	3	NC	1	NC	1
310		min	0	15	-.01	3	0	10	-3.082e-5	10	NC	1	NC	1
311	4	max	0	1	.01	2	0	3	4.448e-4	3	NC	1	NC	1
312		min	0	15	-.009	3	0	10	-3.082e-5	10	NC	1	NC	1
313	5	max	0	1	.01	2	0	3	4.448e-4	3	NC	1	NC	1
314		min	0	15	-.009	3	0	10	-3.082e-5	10	NC	1	NC	1
315	6	max	0	1	.009	2	0	3	4.448e-4	3	NC	1	NC	1
316		min	0	15	-.008	3	0	10	-3.082e-5	10	NC	1	NC	1
317	7	max	0	1	.008	2	0	3	4.448e-4	3	NC	1	NC	1
318		min	0	15	-.007	3	0	10	-3.082e-5	10	NC	1	NC	1
319	8	max	0	1	.008	2	0	3	4.448e-4	3	NC	1	NC	1
320		min	0	15	-.007	3	0	10	-3.082e-5	10	NC	1	NC	1
321	9	max	0	1	.007	2	0	3	4.448e-4	3	NC	1	NC	1
322		min	0	15	-.006	3	0	10	-3.082e-5	10	NC	1	NC	1
323	10	max	0	1	.006	2	0	3	4.448e-4	3	NC	1	NC	1
324		min	0	15	-.006	3	0	10	-3.082e-5	10	NC	1	NC	1
325	11	max	0	1	.006	2	0	3	4.448e-4	3	NC	1	NC	1
326		min	0	15	-.005	3	0	10	-3.082e-5	10	NC	1	NC	1
327	12	max	0	1	.005	2	0	3	4.448e-4	3	NC	1	NC	1
328		min	0	15	-.004	3	0	10	-3.082e-5	10	NC	1	NC	1
329	13	max	0	1	.004	2	0	3	4.448e-4	3	NC	1	NC	1
330		min	0	15	-.004	3	0	10	-3.082e-5	10	NC	1	NC	1
331	14	max	0	1	.003	2	0	3	4.448e-4	3	NC	1	NC	1
332		min	0	15	-.003	3	0	10	-3.082e-5	10	NC	1	NC	1
333	15	max	0	1	.003	2	0	3	4.448e-4	3	NC	1	NC	1
334		min	0	15	-.002	3	0	10	-3.082e-5	10	NC	1	NC	1
335	16	max	0	1	.002	2	0	3	4.448e-4	3	NC	1	NC	1
336		min	0	15	-.002	3	0	10	-3.082e-5	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	3	4.448e-4	3	NC	1	NC	1
338			min	0	15	-.001	3	0	10	-3.082e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	3	4.448e-4	3	NC	1	NC	1
340			min	0	15	0	3	0	10	-3.082e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.448e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-3.082e-5	10	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.005	3	3.921e-3	2	NC	1	NC	1
344			min	-.01	2	-.021	2	0	9	-5.817e-3	3	NC	1	NC	1
345		2	max	.01	3	.016	3	.004	3	1.951e-3	2	NC	4	NC	1
346			min	-.01	2	-.013	2	0	9	-2.859e-3	3	5558.076	2	NC	1
347		3	max	.01	3	.007	3	.003	3	4.48e-5	3	NC	4	NC	1
348			min	-.01	2	-.005	2	0	9	-5.761e-5	9	2818.391	3	NC	1
349		4	max	.01	3	.002	2	.002	3	4.791e-5	3	NC	4	NC	1
350			min	-.01	2	-.002	3	-.001	9	-4.871e-5	9	1888.413	3	NC	1
351		5	max	.01	3	.009	2	.002	3	5.101e-5	3	NC	4	NC	1
352			min	-.01	2	-.008	3	-.001	9	-3.982e-5	9	1478.701	3	NC	1
353		6	max	.01	3	.014	2	.002	3	5.412e-5	3	NC	4	NC	1
354			min	-.01	2	-.014	3	-.001	9	-3.093e-5	9	1257.464	3	NC	1
355		7	max	.01	3	.018	2	.002	3	5.723e-5	3	NC	4	NC	1
356			min	-.01	2	-.018	3	0	9	-2.203e-5	9	1127.604	3	NC	1
357		8	max	.01	3	.021	2	.001	3	6.033e-5	3	NC	4	NC	1
358			min	-.01	2	-.021	3	0	9	-1.314e-5	9	1051.15	3	NC	1
359		9	max	.009	3	.024	2	.001	3	6.344e-5	3	NC	4	NC	1
360			min	-.01	2	-.023	3	0	9	-4.243e-6	9	1011.037	3	NC	1
361		10	max	.009	3	.025	2	.002	3	6.655e-5	3	NC	4	NC	1
362			min	-.01	2	-.024	3	0	9	-2.607e-6	10	999.601	3	NC	1
363		11	max	.009	3	.024	2	.002	3	6.966e-5	3	NC	4	NC	1
364			min	-.01	2	-.023	3	0	10	-6.127e-6	10	1014.586	3	NC	1
365		12	max	.009	3	.023	2	.002	3	7.276e-5	3	NC	4	NC	1
366			min	-.01	2	-.021	3	0	10	-9.648e-6	10	1058.087	3	NC	1
367		13	max	.009	3	.02	2	.002	3	7.587e-5	3	NC	4	NC	1
368			min	-.01	2	-.018	3	0	10	-1.317e-5	10	1137.416	3	NC	1
369		14	max	.009	3	.015	2	.002	3	7.898e-5	3	NC	4	NC	1
370			min	-.01	2	-.014	3	0	10	-1.669e-5	10	1268.658	3	NC	1
371		15	max	.009	3	.009	2	.002	3	8.208e-5	3	NC	4	NC	1
372			min	-.01	2	-.008	3	0	10	-2.021e-5	10	1486.735	3	NC	1
373		16	max	.009	3	.002	2	.002	3	8.626e-5	3	NC	4	NC	1
374			min	-.01	2	-.002	3	0	10	-2.279e-5	10	1877.52	3	NC	1
375		17	max	.009	3	.006	3	.002	3	1.158e-4	3	NC	4	NC	1
376			min	-.01	2	-.008	2	0	10	-2.862e-6	10	2724.254	3	NC	1
377		18	max	.009	3	.014	3	.002	3	2.894e-3	2	NC	1	NC	1
378			min	-.01	2	-.018	2	0	10	-1.711e-3	3	5341.638	3	NC	1
379		19	max	.009	3	.023	3	.001	3	5.843e-3	2	NC	1	NC	1
380			min	-.01	2	-.029	2	0	9	-3.572e-3	3	5736.633	2	NC	1
381	M5	1	max	.026	3	.076	3	.005	3	2.195e-5	3	NC	1	NC	1
382			min	-.027	2	-.061	2	0	9	0	15	4204.935	3	NC	1
383		2	max	.026	3	.046	3	.007	3	1.757e-4	3	NC	4	NC	1
384			min	-.027	2	-.037	2	0	9	-2.069e-6	9	1914.723	2	NC	1
385		3	max	.026	3	.018	3	.008	3	3.263e-4	3	NC	4	NC	1
386			min	-.027	2	-.014	2	0	9	-4.172e-6	9	981.355	2	NC	1
387		4	max	.026	3	.007	2	.009	3	3.128e-4	3	NC	5	NC	1
388			min	-.027	2	-.006	3	0	9	-3.901e-6	9	664.131	3	9782.011	3
389		5	max	.026	3	.025	2	.01	3	2.992e-4	3	NC	5	NC	1
390			min	-.027	2	-.026	3	0	9	-3.629e-6	9	519.191	3	8198.947	3
391		6	max	.026	3	.041	2	.01	3	2.856e-4	3	NC	5	NC	1
392			min	-.027	2	-.041	3	0	9	-3.358e-6	9	441.597	3	7422.814	3
393		7	max	.025	3	.053	2	.01	3	2.72e-4	3	NC	5	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394		min	-.027	2	-.054	3	0	9	-3.087e-6	9	396.404	3	7080.451	3
395	8	max	.025	3	.063	2	.01	3	2.585e-4	3	NC	5	NC	1
396		min	-.027	2	-.062	3	0	9	-2.815e-6	9	370.081	3	7030.076	3
397	9	max	.025	3	.069	2	.01	3	2.449e-4	3	NC	5	NC	1
398		min	-.027	2	-.067	3	0	9	-2.544e-6	9	356.587	3	7220.608	3
399	10	max	.025	3	.071	2	.009	3	2.313e-4	3	NC	5	NC	1
400		min	-.027	2	-.068	3	0	9	-2.272e-6	9	352.662	2	7650.516	3
401	11	max	.025	3	.071	2	.008	3	2.177e-4	3	NC	5	NC	1
402		min	-.027	2	-.066	3	0	9	-2.001e-6	9	355.116	2	8358.72	3
403	12	max	.025	3	.066	2	.008	3	2.041e-4	3	NC	5	NC	1
404		min	-.027	2	-.06	3	0	9	-1.729e-6	9	368.338	2	9433.487	3
405	13	max	.025	3	.057	2	.007	3	1.906e-4	3	NC	5	NC	1
406		min	-.027	2	-.051	3	0	9	-1.458e-6	9	395.509	2	NC	1
407	14	max	.025	3	.044	2	.006	3	1.77e-4	3	NC	5	NC	1
408		min	-.027	2	-.039	3	0	9	-1.186e-6	9	443.947	2	NC	1
409	15	max	.025	3	.027	2	.005	3	1.634e-4	3	NC	5	NC	1
410		min	-.027	2	-.024	3	0	9	-9.147e-7	9	530.603	3	NC	1
411	16	max	.025	3	.005	2	.004	3	1.455e-4	3	NC	5	NC	1
412		min	-.027	2	-.005	3	0	9	-9.001e-7	9	670.97	3	NC	1
413	17	max	.025	3	.017	3	.003	3	2.389e-5	3	NC	4	NC	1
414		min	-.027	2	-.022	2	0	9	-7.001e-6	9	973.179	3	NC	1
415	18	max	.025	3	.04	3	.002	3	8.975e-6	3	NC	4	NC	1
416		min	-.027	2	-.053	2	0	9	-3.658e-6	9	1908.322	3	NC	1
417	19	max	.025	3	.065	3	.001	3	-5.056e-8	15	NC	3	NC	1
418		min	-.027	2	-.085	2	0	9	-3.933e-6	3	1967.104	2	NC	1
419	M9	1	max	.01	.025	3	.005	3	5.861e-3	3	NC	1	NC	1
420		min	-.01	2	-.021	2	0	9	-3.921e-3	2	NC	1	NC	1
421	2	max	.01	3	.015	3	.003	3	2.867e-3	3	NC	4	NC	1
422		min	-.01	2	-.013	2	0	10	-1.951e-3	2	4617.154	3	NC	1
423	3	max	.01	3	.005	3	0	3	5.554e-5	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	10	-7.081e-5	3	2388.287	3	NC	1
425	4	max	.01	3	.002	2	.001	1	4.637e-5	1	NC	4	NC	1
426		min	-.01	2	-.003	3	0	3	-7.398e-5	3	1696.708	3	NC	1
427	5	max	.01	3	.009	2	.001	1	3.72e-5	1	NC	4	NC	1
428		min	-.01	2	-.01	3	-.002	3	-7.715e-5	3	1365.979	3	8252.455	3
429	6	max	.01	3	.014	2	.001	1	2.803e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.003	3	-8.032e-5	3	1180.544	3	7150.266	3
431	7	max	.01	3	.018	2	0	1	1.886e-5	1	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-8.348e-5	3	1069.876	3	6509.039	3
433	8	max	.01	3	.021	2	0	1	9.694e-6	1	NC	4	NC	1
434		min	-.01	2	-.022	3	-.004	3	-8.665e-5	3	1004.775	3	6146.85	3
435	9	max	.01	3	.024	2	0	1	5.253e-7	1	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-8.982e-5	3	971.76	3	5981.275	3
437	10	max	.01	3	.025	2	0	1	2.744e-6	10	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-9.299e-5	3	964.83	3	5976.602	3
439	11	max	.01	3	.024	2	0	10	6.255e-6	10	NC	4	NC	1
440		min	-.01	2	-.023	3	-.005	3	-9.615e-5	3	982.558	3	6125.451	3
441	12	max	.009	3	.023	2	0	10	9.767e-6	10	NC	4	NC	1
442		min	-.01	2	-.021	3	-.005	3	-9.932e-5	3	1027.436	3	6444.669	3
443	13	max	.009	3	.02	2	0	10	1.328e-5	10	NC	4	NC	1
444		min	-.01	2	-.018	3	-.004	3	-1.025e-4	3	1106.89	3	6981.462	3
445	14	max	.009	3	.015	2	0	10	1.679e-5	10	NC	4	NC	1
446		min	-.01	2	-.014	3	-.004	3	-1.057e-4	3	1236.842	3	7834.665	3
447	15	max	.009	3	.009	2	0	10	2.03e-5	10	NC	4	NC	1
448		min	-.01	2	-.008	3	-.003	3	-1.088e-4	3	1451.606	3	9212.565	3
449	16	max	.009	3	.002	2	0	10	2.286e-5	10	NC	4	NC	1
450		min	-.01	2	-.002	3	-.002	3	-1.009e-4	3	1835.338	3	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.009	3	.006	3	0	10	1.714e-4	3	NC	4	NC	1
452		min	-.01	2	-.008	2	-.001	3	-1.545e-5	9	2665.061	3	NC	1
453	18	max	.009	3	.015	3	0	10	1.855e-3	3	NC	1	NC	1
454		min	-.01	2	-.018	2	0	9	-2.894e-3	2	5227.823	3	NC	1
455	19	max	.009	3	.024	3	.001	3	3.564e-3	3	NC	1	NC	1
456		min	-.01	2	-.029	2	0	9	-5.844e-3	2	5750.217	2	NC	1
457	M13	1	max	0	.025	3	.01	3	3.875e-3	3	NC	1	NC	1
458		min	-.005	3	-.021	2	-.01	2	-3.204e-3	2	NC	1	NC	1
459	2	max	0	9	.048	3	.008	3	4.709e-3	3	NC	4	NC	1
460		min	-.005	3	-.037	2	-.01	2	-3.882e-3	2	3170.313	3	NC	1
461	3	max	0	9	.068	3	.008	3	5.542e-3	3	NC	4	NC	1
462		min	-.005	3	-.05	2	-.011	2	-4.559e-3	2	1694.997	3	NC	1
463	4	max	0	9	.083	3	.01	3	6.376e-3	3	NC	4	NC	1
464		min	-.005	3	-.06	2	-.013	2	-5.236e-3	2	1257.513	3	NC	1
465	5	max	0	9	.091	3	.012	3	7.209e-3	3	NC	4	NC	1
466		min	-.005	3	-.067	2	-.015	2	-5.913e-3	2	1091.273	3	NC	1
467	6	max	0	9	.094	3	.014	3	8.043e-3	3	NC	4	NC	1
468		min	-.005	3	-.07	2	-.018	2	-6.59e-3	2	1050.216	3	8582.813	2
469	7	max	0	9	.091	3	.017	3	8.876e-3	3	NC	4	NC	1
470		min	-.005	3	-.069	2	-.021	2	-7.268e-3	2	1092.793	3	6267.259	2
471	8	max	0	9	.085	3	.02	3	9.71e-3	3	NC	4	NC	1
472		min	-.005	3	-.067	2	-.024	2	-7.945e-3	2	1203.278	3	4985.909	2
473	9	max	0	9	.079	3	.023	3	1.054e-2	3	NC	4	NC	4
474		min	-.005	3	-.063	2	-.027	2	-8.622e-3	2	1351.871	3	4307.176	2
475	10	max	0	9	.076	3	.026	3	1.138e-2	3	NC	4	NC	4
476		min	-.005	3	-.061	2	-.027	2	-9.299e-3	2	1440.405	3	4078.545	2
477	11	max	0	9	.079	3	.027	3	1.055e-2	3	NC	4	NC	4
478		min	-.005	3	-.063	2	-.027	2	-8.622e-3	2	1351.87	3	4125.315	3
479	12	max	0	9	.086	3	.027	3	9.719e-3	3	NC	4	NC	1
480		min	-.005	3	-.067	2	-.024	2	-7.945e-3	2	1203.277	3	4123.779	3
481	13	max	0	9	.092	3	.026	3	8.89e-3	3	NC	4	NC	1
482		min	-.005	3	-.069	2	-.021	2	-7.268e-3	2	1092.792	3	4421.879	3
483	14	max	0	9	.095	3	.024	3	8.061e-3	3	NC	4	NC	1
484		min	-.005	3	-.07	2	-.018	2	-6.59e-3	2	1050.215	3	5079.295	3
485	15	max	0	9	.092	3	.021	3	7.232e-3	3	NC	4	NC	1
486		min	-.005	3	-.067	2	-.015	2	-5.913e-3	2	1091.271	3	6309.563	3
487	16	max	0	9	.084	3	.018	3	6.403e-3	3	NC	4	NC	1
488		min	-.005	3	-.06	2	-.013	2	-5.236e-3	2	1257.511	3	8689.715	3
489	17	max	0	9	.069	3	.015	3	5.575e-3	3	NC	4	NC	1
490		min	-.005	3	-.05	2	-.011	2	-4.559e-3	2	1694.995	3	NC	1
491	18	max	0	9	.049	3	.012	3	4.746e-3	3	NC	4	NC	1
492		min	-.005	3	-.037	2	-.01	2	-3.882e-3	2	3170.309	3	NC	1
493	19	max	0	9	.027	3	.01	3	3.917e-3	3	NC	1	NC	1
494		min	-.005	3	-.021	2	-.01	2	-3.205e-3	2	NC	1	NC	1
495	M16	1	max	0	.024	3	.009	3	4.259e-3	2	NC	1	NC	1
496		min	-.001	3	-.029	2	-.01	2	-3.383e-3	3	NC	1	NC	1
497	2	max	0	9	.037	3	.012	3	5.16e-3	2	NC	4	NC	1
498		min	-.001	3	-.052	2	-.01	2	-4.046e-3	3	3162.337	2	NC	1
499	3	max	0	9	.05	3	.014	3	6.062e-3	2	NC	4	NC	1
500		min	-.001	3	-.072	2	-.011	2	-4.709e-3	3	1684.602	2	NC	1
501	4	max	0	9	.06	3	.017	3	6.963e-3	2	NC	4	NC	1
502		min	-.001	3	-.087	2	-.013	2	-5.372e-3	3	1242.198	2	9269.287	3
503	5	max	0	9	.066	3	.02	3	7.865e-3	2	NC	4	NC	1
504		min	-.001	3	-.097	2	-.015	2	-6.035e-3	3	1068.299	2	6939.224	3
505	6	max	0	9	.07	3	.022	3	8.766e-3	2	NC	4	NC	1
506		min	-.001	3	-.1	2	-.018	2	-6.698e-3	3	1015.127	2	5689.133	3
507	7	max	0	9	.07	3	.024	3	9.668e-3	2	NC	4	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	-0.001	3	-0.099	2	-0.021	2	-7.361e-3	3	1038.173	2	4987.565	3
509	8	max	0	9	.068	3	.025	3	1.057e-2	2	NC	4	NC	1
510		min	-0.001	3	-0.094	2	-0.024	2	-8.024e-3	3	1118.157	2	4625.513	3
511	9	max	0	9	.066	3	.025	3	1.147e-2	2	NC	4	NC	4
512		min	-0.001	3	-0.088	2	-0.026	2	-8.687e-3	3	1226.981	2	4329.003	2
513	10	max	0	9	.065	3	.025	3	1.237e-2	2	NC	4	NC	4
514		min	-0.001	3	-0.085	2	-0.027	2	-9.35e-3	3	1290.92	2	4098.569	2
515	11	max	0	9	.066	3	.023	3	1.147e-2	2	NC	4	NC	4
516		min	-0.001	3	-0.088	2	-0.026	2	-8.682e-3	3	1226.981	2	4329.007	2
517	12	max	0	9	.068	3	.022	3	1.057e-2	2	NC	4	NC	1
518		min	-0.001	3	-0.094	2	-0.024	2	-8.015e-3	3	1118.157	2	5013.301	2
519	13	max	0	9	.07	3	.02	3	9.668e-3	2	NC	4	NC	1
520		min	-0.001	3	-0.099	2	-0.021	2	-7.347e-3	3	1038.173	2	6306.074	2
521	14	max	0	9	.07	3	.018	3	8.767e-3	2	NC	4	NC	1
522		min	-0.001	3	-.1	2	-0.018	2	-6.679e-3	3	1015.127	2	8198.594	3
523	15	max	0	9	.066	3	.016	3	7.866e-3	2	NC	4	NC	1
524		min	-0.001	3	-0.097	2	-0.015	2	-6.012e-3	3	1068.299	2	NC	1
525	16	max	0	9	.06	3	.014	3	6.964e-3	2	NC	4	NC	1
526		min	-0.001	3	-0.087	2	-0.013	2	-5.344e-3	3	1242.198	2	NC	1
527	17	max	0	9	.05	3	.012	3	6.063e-3	2	NC	4	NC	1
528		min	-0.001	3	-0.072	2	-0.011	2	-4.677e-3	3	1684.602	2	NC	1
529	18	max	0	9	.037	3	.01	3	5.162e-3	2	NC	4	NC	1
530		min	-0.001	3	-0.052	2	-.01	2	-4.009e-3	3	3162.337	2	NC	1
531	19	max	0	9	.023	3	.009	3	4.26e-3	2	NC	1	NC	1
532		min	-0.001	3	-0.029	2	-.01	2	-3.342e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.968e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-3.169e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.052e-4	3	NC	1	NC	1
536		min	0	2	-0.002	4	0	3	-3.319e-4	2	NC	1	NC	1
537	3	max	0	3	0	15	.002	2	1.014e-3	3	NC	1	NC	1
538		min	0	2	-0.003	4	-0.003	3	-6.322e-4	2	NC	1	8693.889	3
539	4	max	0	3	-0.001	15	.005	2	1.322e-3	3	NC	1	NC	4
540		min	0	2	-0.004	4	-0.006	3	-9.324e-4	2	NC	1	4803.256	3
541	5	max	0	3	-0.001	15	.008	2	1.63e-3	3	NC	1	NC	4
542		min	0	2	-0.006	4	-.01	3	-1.233e-3	2	9790.17	4	3155.807	3
543	6	max	0	3	-0.002	15	.012	2	1.939e-3	3	NC	1	NC	4
544		min	-0.001	2	-0.007	4	-0.015	3	-1.533e-3	2	8239.463	4	2299.466	3
545	7	max	0	3	-0.002	15	.016	2	2.247e-3	3	NC	2	NC	4
546		min	-0.001	2	-0.008	4	-.02	3	-1.833e-3	2	7306.917	4	1798.49	3
547	8	max	0	3	-0.002	15	.019	2	2.556e-3	3	NC	2	NC	4
548		min	-0.002	2	-0.008	4	-0.025	3	-2.133e-3	2	6747.249	4	1483.46	3
549	9	max	.001	3	-0.002	2	.023	2	2.864e-3	3	NC	2	NC	4
550		min	-0.002	2	-0.009	4	-0.029	3	-2.434e-3	2	6446.003	4	1277.263	3
551	10	max	.001	3	-0.001	2	.025	2	3.172e-3	3	NC	2	NC	4
552		min	-0.002	2	-0.009	4	-0.033	3	-2.734e-3	2	6350.703	4	1141.157	3
553	11	max	.001	3	0	2	.027	2	3.481e-3	3	NC	2	NC	4
554		min	-0.002	2	-0.009	4	-0.035	3	-3.034e-3	2	6446.003	4	1054.801	3
555	12	max	.002	3	0	2	.027	2	3.789e-3	3	NC	2	NC	4
556		min	-0.003	2	-0.008	4	-0.035	3	-3.334e-3	2	6747.249	4	1008.106	3
557	13	max	.002	3	.001	2	.026	2	4.097e-3	3	NC	2	NC	4
558		min	-0.003	2	-0.008	4	-0.034	3	-3.634e-3	2	7306.917	4	998.152	3
559	14	max	.002	3	.002	2	.023	2	4.406e-3	3	NC	1	NC	4
560		min	-0.003	2	-0.007	4	-0.031	3	-3.935e-3	2	8239.463	4	1029.304	3
561	15	max	.002	3	.004	2	.018	2	4.714e-3	3	NC	1	NC	4
562		min	-0.003	2	-0.006	4	-0.025	3	-4.235e-3	2	9790.17	4	1117.564	3
563	16	max	.002	3	.005	2	.011	1	5.023e-3	3	NC	1	NC	4
564		min	-0.004	2	-0.005	3	-0.016	3	-4.535e-3	2	NC	1	1306.372	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.002	3	.007	2	.004	1	5.331e-3	3	NC	1	NC	4
566			min	-.004	2	-.004	3	-.003	3	-4.835e-3	2	7825.388	2	1732.007	3
567		18	max	.002	3	.009	2	.013	3	5.639e-3	3	NC	1	NC	4
568			min	-.004	2	-.003	3	-.013	2	-5.136e-3	2	6274.877	2	3083.857	3
569		19	max	.002	3	.011	2	.032	3	5.948e-3	3	NC	1	NC	1
570			min	-.004	2	-.002	3	-.029	2	-5.436e-3	2	5216.547	2	NC	1
571	M16A	1	max	.002	2	.004	2	.01	3	1.693e-3	3	NC	1	NC	1
572			min	-.002	3	-.004	3	-.01	2	-1.899e-3	2	NC	1	NC	1
573		2	max	.002	2	.003	2	.003	3	1.636e-3	3	NC	1	NC	1
574			min	-.002	3	-.005	3	-.005	2	-1.811e-3	2	NC	1	8375.747	3
575		3	max	.001	2	.001	2	.001	9	1.579e-3	3	NC	1	NC	4
576			min	-.002	3	-.006	3	-.003	3	-1.722e-3	2	NC	1	4747.087	3
577		4	max	.001	2	0	2	.004	1	1.523e-3	3	NC	1	NC	4
578			min	-.002	3	-.006	3	-.007	3	-1.634e-3	2	NC	1	3617.551	3
579		5	max	.001	2	-.001	2	.005	1	1.466e-3	3	NC	1	NC	4
580			min	-.002	3	-.007	3	-.01	3	-1.546e-3	2	9790.17	4	3131.296	3
581		6	max	.001	2	-.002	15	.006	1	1.409e-3	3	NC	1	NC	4
582			min	-.002	3	-.007	4	-.012	3	-1.457e-3	2	8239.463	4	2923.33	3
583		7	max	.001	2	-.002	15	.007	2	1.352e-3	3	NC	3	NC	4
584			min	-.002	3	-.008	4	-.013	3	-1.369e-3	2	7306.917	4	2879.954	3
585		8	max	.001	2	-.002	15	.007	2	1.295e-3	3	NC	3	NC	4
586			min	-.001	3	-.009	4	-.013	3	-1.28e-3	2	6747.249	4	2963.37	3
587		9	max	0	2	-.002	15	.007	2	1.238e-3	3	NC	3	NC	4
588			min	-.001	3	-.009	4	-.012	3	-1.192e-3	2	6446.003	4	3170.641	3
589		10	max	0	2	-.002	15	.006	2	1.182e-3	3	NC	3	NC	4
590			min	-.001	3	-.009	4	-.011	3	-1.103e-3	2	6350.703	4	3524.994	3
591		11	max	0	2	-.002	15	.005	1	1.125e-3	3	NC	3	NC	4
592			min	-.001	3	-.009	4	-.009	3	-1.015e-3	2	6446.003	4	4081.989	3
593		12	max	0	2	-.002	15	.004	1	1.068e-3	3	NC	3	NC	4
594			min	0	3	-.008	4	-.007	3	-9.265e-4	2	6747.249	4	4952.833	3
595		13	max	0	2	-.002	15	.003	1	1.011e-3	3	NC	3	NC	1
596			min	0	3	-.008	4	-.005	3	-8.381e-4	2	7306.917	4	6365.081	3
597		14	max	0	2	-.002	15	.002	1	9.543e-4	3	NC	1	NC	1
598			min	0	3	-.007	4	-.003	3	-7.496e-4	2	8239.463	4	8831.834	3
599		15	max	0	2	-.001	15	.001	4	8.975e-4	3	NC	1	NC	1
600			min	0	3	-.006	4	-.002	3	-6.612e-4	2	9790.17	4	NC	1
601		16	max	0	2	-.001	15	.001	4	8.406e-4	3	NC	1	NC	1
602			min	0	3	-.005	4	0	3	-5.728e-4	2	NC	1	NC	1
603		17	max	0	2	0	15	0	4	7.838e-4	3	NC	1	NC	1
604			min	0	3	-.003	4	0	2	-4.843e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	3	7.27e-4	3	NC	1	NC	1
606			min	0	3	-.002	4	0	2	-3.959e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	6.701e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.075e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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