

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	96 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-2.049 k-ft
$M_z$ =	0.005 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>74%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

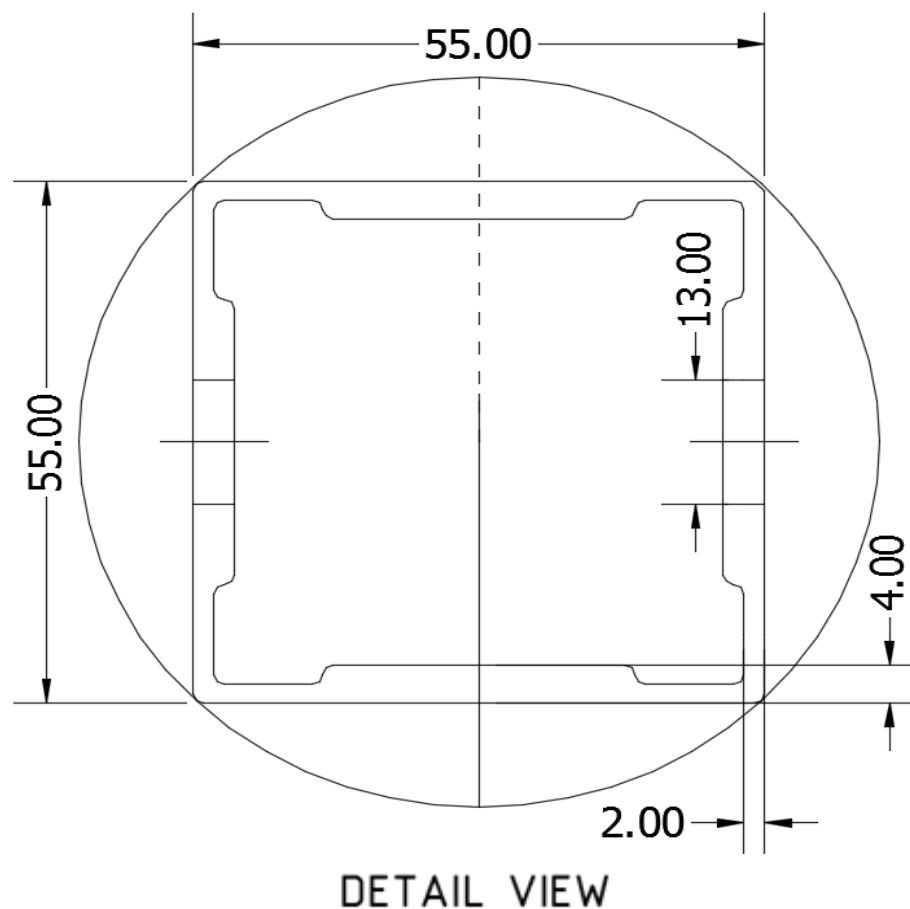
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.991 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.934 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>88%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

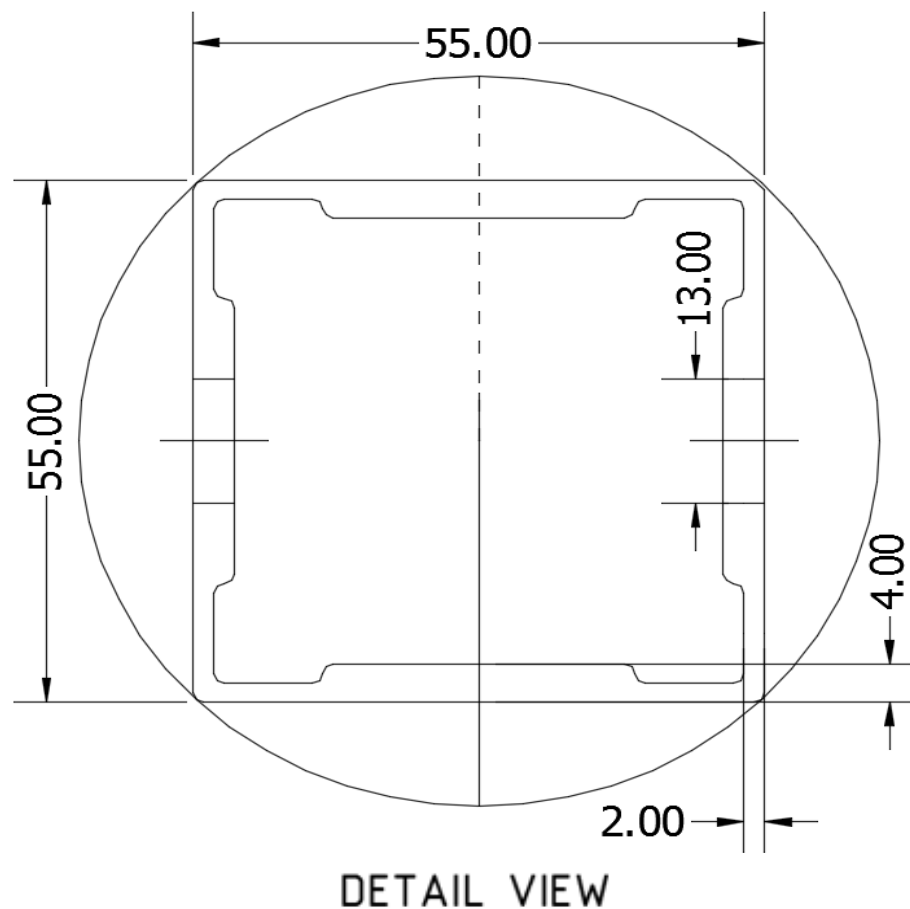
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.440 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>9%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.705 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>37%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>70.83</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	10.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.238 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.365 k
Utilization =	<u>32%</u>



#### 5. FOUNDATION DESIGN CALCULATIONS

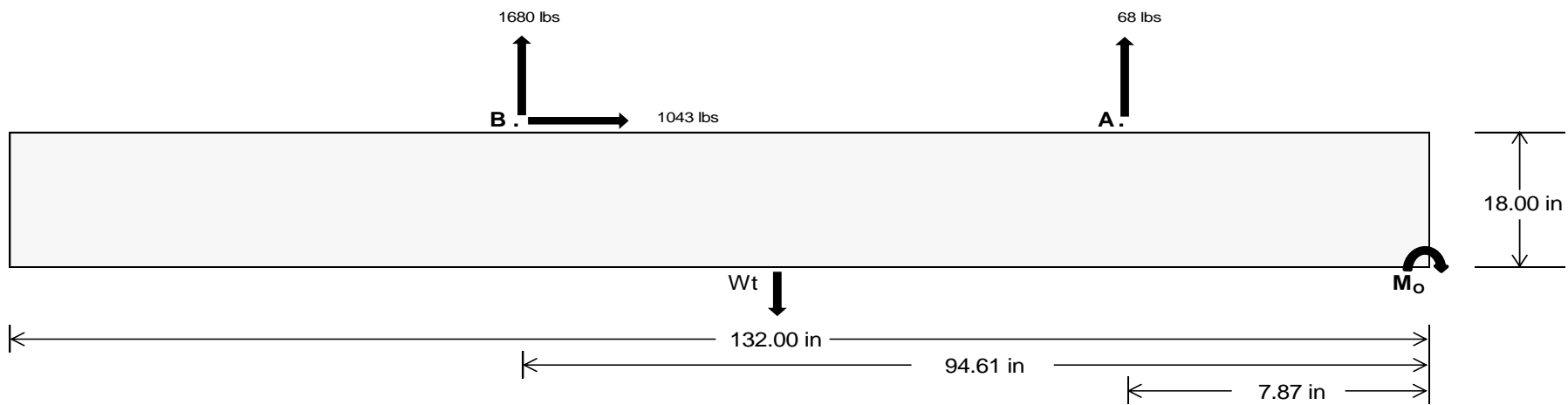
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>295.39</u>	<u>6995.06</u> k
Compressive Load =	<u>3171.92</u>	<u>5143.36</u> k
Lateral Load =	<u>9.38</u>	<u>4340.47</u> k
Moment (Weak Axis) =	<u>0.02</u>	<u>0.00</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 178219.4$  in-lbs  
Resisting Force Required = 2700.29 lbs  
S.F. = 1.67  
Weight Required = 4500.49 lbs  
Minimum Width = **36 in** in  
Weight Provided = 7177.50 lbs

### Sliding

Force = 1043.33 lbs  
Friction = 0.4  
Weight Required = 2608.34 lbs  
Resisting Weight = 7177.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 1043.33 lbs  
Cohesion = 130 psf  
Area = 33.00 ft<sup>2</sup>  
Resisting = 3588.75 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 132in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 36in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 36in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

	Ballast Width			
	36 in	37 in	38 in	39 in
$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(3 \text{ ft}) =$	7178 lbs	7377 lbs	7576 lbs	7776 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in
$F_A$	985 lbs	985 lbs	985 lbs	985 lbs	1337 lbs	1337 lbs	1337 lbs	1337 lbs	1635 lbs	1635 lbs	1635 lbs	1635 lbs	-137 lbs	-137 lbs	-137 lbs	-137 lbs
$F_B$	927 lbs	927 lbs	927 lbs	927 lbs	2294 lbs	2294 lbs	2294 lbs	2294 lbs	2316 lbs	2316 lbs	2316 lbs	2316 lbs	-3359 lbs	-3359 lbs	-3359 lbs	-3359 lbs
$F_V$	138 lbs	138 lbs	138 lbs	138 lbs	1876 lbs	1876 lbs	1876 lbs	1876 lbs	1497 lbs	1497 lbs	1497 lbs	1497 lbs	-2087 lbs	-2087 lbs	-2087 lbs	-2087 lbs
$P_{total}$	9090 lbs	9289 lbs	9488 lbs	9688 lbs	10809 lbs	11008 lbs	11208 lbs	11407 lbs	11129 lbs	11328 lbs	11528 lbs	11727 lbs	811 lbs	930 lbs	1050 lbs	1169 lbs
$M$	2768 lbs-ft	2768 lbs-ft	2768 lbs-ft	2768 lbs-ft	3821 lbs-ft	3821 lbs-ft	3821 lbs-ft	3821 lbs-ft	4644 lbs-ft	4644 lbs-ft	4644 lbs-ft	4644 lbs-ft	4216 lbs-ft	4216 lbs-ft	4216 lbs-ft	4216 lbs-ft
$e$	0.30 ft	0.30 ft	0.29 ft	0.29 ft	0.35 ft	0.35 ft	0.34 ft	0.33 ft	0.42 ft	0.41 ft	0.40 ft	0.40 ft	5.20 ft	4.53 ft	4.02 ft	3.60 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	229.7 psf	229.4 psf	229.0 psf	228.7 psf	264.4 psf	263.1 psf	261.9 psf	260.8 psf	260.5 psf	259.3 psf	258.2 psf	257.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	321.2 psf	318.4 psf	315.7 psf	313.2 psf	390.7 psf	386.0 psf	381.6 psf	377.4 psf	414.0 psf	408.7 psf	403.7 psf	398.9 psf	602.0 psf	207.8 psf	148.9 psf	126.6 psf

Maximum Bearing Pressure = 602 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

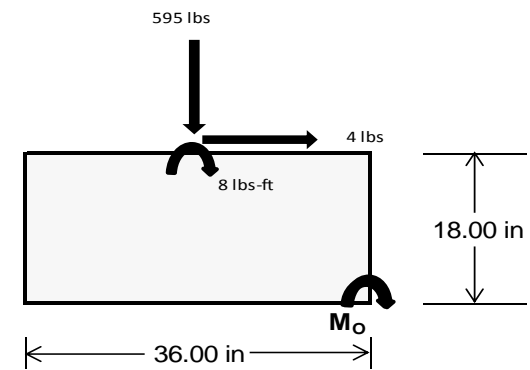
#### Overturning Check

$M_O = 878.3 \text{ ft-lbs}$   
 Resisting Force Required = 585.51 lbs  
 S.F. = 1.67  
 Weight Required = 975.86 lbs  
 Minimum Width = 36 in  
 Weight Provided = 7177.50 lbs

*A minimum 132in long x 36in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	36 in			36 in			36 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_Y$	211 lbs	510 lbs	211 lbs	595 lbs	1611 lbs	595 lbs	62 lbs	149 lbs	62 lbs
$F_V$	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
$P_{\text{total}}$	9097 lbs	7178 lbs	9097 lbs	9053 lbs	7178 lbs	9053 lbs	2660 lbs	7178 lbs	2660 lbs
$M$	4 lbs-ft	0 lbs-ft	4 lbs-ft	14 lbs-ft	0 lbs-ft	14 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft
$f_{\text{min}}$	275.4 psf	217.5 psf	275.4 psf	273.5 psf	217.5 psf	273.5 psf	80.6 psf	217.5 psf	80.6 psf
$f_{\text{max}}$	275.9 psf	217.5 psf	275.9 psf	275.2 psf	217.5 psf	275.2 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 276 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

### 5.3 Foundation Anchors

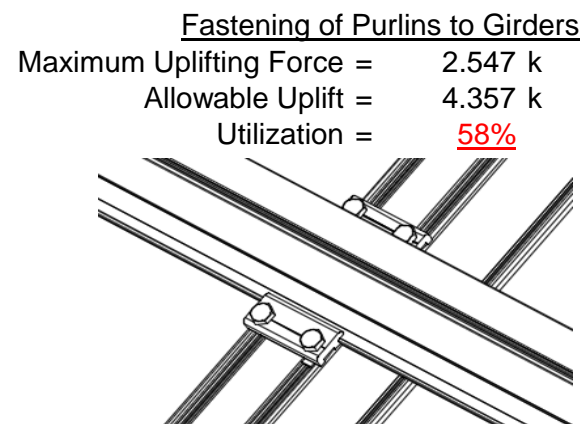
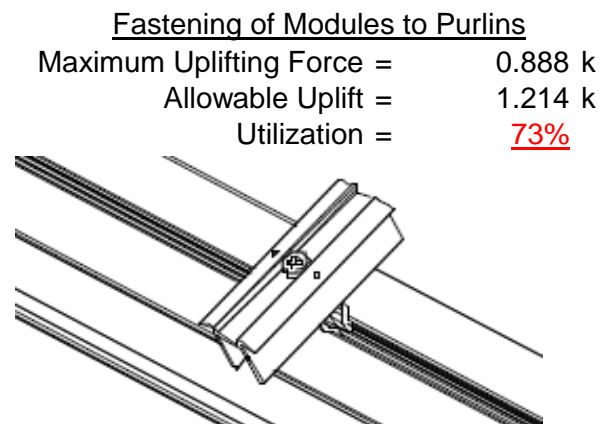
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

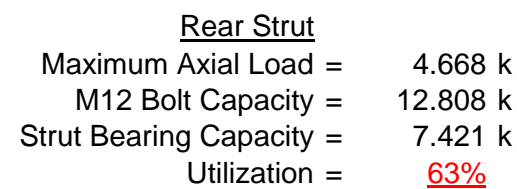
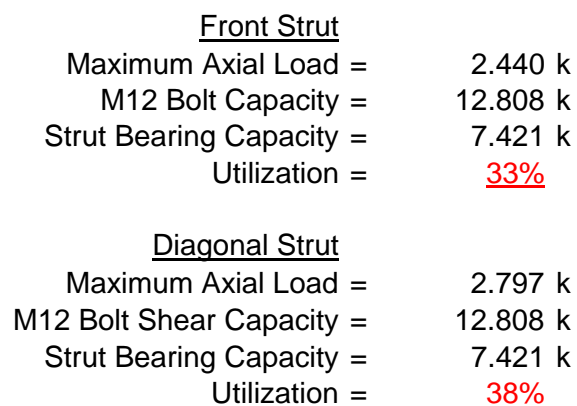
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



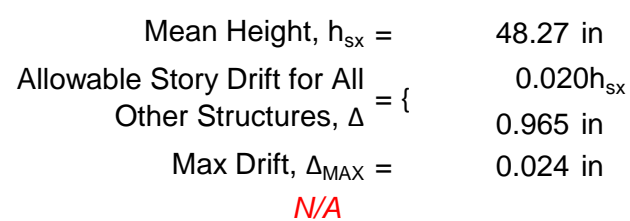
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

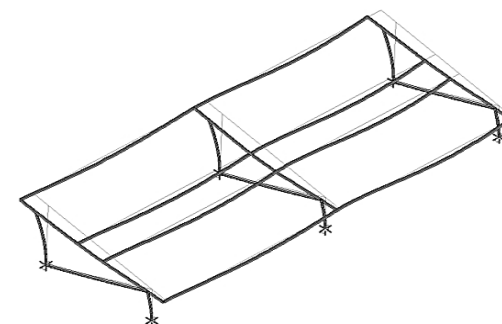
## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 96 \text{ in}$$

$$J = 0.432$$

$$265.581$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 96$$

$$J = 0.432$$

$$168.894$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.1$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 70.83 \text{ in} \\ J &= 0.942 \\ &= 110.537 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 70.83 \\ J &= 0.942 \\ &= 110.537 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.63853$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.80939$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 10.5516 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 10.55 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 10.86 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-39.836	-39.836	0	0
2	M14	Y	-39.836	-39.836	0	0
3	M15	Y	-39.836	-39.836	0	0
4	M16	Y	-39.836	-39.836	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-72.509	-72.509	0	0
2	M14	y	-72.509	-72.509	0	0
3	M15	y	-116.645	-116.645	0	0
4	M16	y	-116.645	-116.645	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	163.933	163.933	0	0
2	M14	y	126.102	126.102	0	0
3	M15	y	69.356	69.356	0	0
4	M16	y	69.356	69.356	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



RISA-3D Version 13.0.0 [T:\... \PVMMax 60 Cell 2V 30° 120mph 30psf 8ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	68.017	1	215.782	2	.873	3	.014	2	-.005	15	.898	3
28			min	3.287	15	-370.577	3	-24.504	1	0	15	-.102	1	-.458	2
29		15	max	68.017	1	88.207	2	9.087	1	.014	2	-.005	12	1.125	3
30			min	3.287	15	-141.553	3	.108	10	0	15	-.109	1	-.594	2
31		16	max	68.017	1	87.471	3	42.679	1	.014	2	-.002	12	1.149	3
32			min	3.287	15	-39.367	2	2.063	15	0	15	-.086	1	-.615	2
33		17	max	68.017	1	316.495	3	76.271	1	.014	2	.003	3	.97	3
34			min	3.287	15	-166.941	2	3.651	15	0	15	-.033	1	-.524	2
35		18	max	68.017	1	545.519	3	109.862	1	.014	2	.049	1	.587	3
36			min	3.287	15	-294.515	2	5.239	15	0	15	.002	15	-.318	2
37		19	max	68.017	1	774.543	3	143.454	1	.014	2	.162	1	0	2
38			min	3.287	15	-422.09	2	6.827	15	0	15	.008	15	0	3
39	M14	1	max	36.315	1	471.241	2	-7.08	15	.011	3	.19	1	0	1
40			min	1.75	15	-623.613	3	-148.775	1	-.012	2	.009	15	0	3
41		2	max	36.315	1	343.666	2	-5.492	15	.011	3	.073	1	.476	3
42			min	1.75	15	-448.393	3	-115.183	1	-.012	2	.004	15	-.362	2
43		3	max	36.315	1	216.092	2	-3.905	15	.011	3	.005	3	.797	3
44			min	1.75	15	-273.174	3	-81.592	1	-.012	2	-.015	1	-.611	2
45		4	max	36.315	1	88.518	2	-2.317	15	.011	3	-.001	12	.962	3
46			min	1.75	15	-97.954	3	-.48	1	-.012	2	-.072	1	-.746	2
47		5	max	36.315	1	77.266	3	-.729	15	.011	3	-.004	12	.971	3
48			min	1.75	15	-39.056	2	-14.408	1	-.012	2	-.1	1	-.768	2
49		6	max	36.315	1	252.486	3	19.183	1	.011	3	-.005	15	.825	3
50			min	1.75	15	-166.63	2	-1.266	3	-.012	2	-.098	1	-.677	2
51		7	max	36.315	1	427.706	3	52.775	1	.011	3	-.003	15	.522	3
52			min	1.75	15	-294.205	2	.848	12	-.012	2	-.066	1	-.472	2
53		8	max	36.315	1	602.926	3	86.366	1	.011	3	.002	10	.064	3
54			min	1.75	15	-421.779	2	2.436	12	-.012	2	-.006	3	-.154	2
55		9	max	36.315	1	778.146	3	119.958	1	.011	3	.088	1	.278	2
56			min	1.75	15	-549.353	2	4.024	12	-.012	2	-.002	3	-.549	3
57		10	max	36.315	1	676.927	2	-5.611	12	.011	3	.209	1	.823	2
58			min	1.75	15	-953.366	3	-153.549	1	-.012	2	.003	12	-1.319	3
59		11	max	36.315	1	549.353	2	-4.024	12	.012	2	.088	1	.278	2
60			min	1.75	15	-778.146	3	-119.958	1	-.011	3	-.002	3	-.549	3
61		12	max	36.315	1	421.779	2	-2.436	12	.012	2	.002	10	.064	3
62			min	1.75	15	-602.926	3	-86.366	1	-.011	3	-.006	3	-.154	2
63		13	max	36.315	1	294.205	2	-.848	12	.012	2	-.003	15	.522	3
64			min	1.75	15	-427.706	3	-52.775	1	-.011	3	-.066	1	-.472	2
65		14	max	36.315	1	166.63	2	1.266	3	.012	2	-.005	15	.825	3
66			min	1.75	15	-252.486	3	-19.183	1	-.011	3	-.098	1	-.677	2
67		15	max	36.315	1	39.056	2	14.408	1	.012	2	-.004	12	.971	3
68			min	1.75	15	-77.266	3	.729	15	-.011	3	-.1	1	-.768	2
69		16	max	36.315	1	97.954	3	.48	1	.012	2	-.001	12	.962	3
70			min	1.75	15	-88.518	2	2.317	15	-.011	3	-.072	1	-.746	2
71		17	max	36.315	1	273.174	3	81.592	1	.012	2	.005	3	.797	3
72			min	1.75	15	-216.092	2	3.905	15	-.011	3	-.015	1	-.611	2
73		18	max	36.315	1	448.393	3	115.183	1	.012	2	.073	1	.476	3
74			min	1.75	15	-343.666	2	5.492	15	-.011	3	.004	15	-.362	2
75		19	max	36.315	1	623.613	3	148.775	1	.012	2	.19	1	0	1
76			min	1.75	15	-471.241	2	7.08	15	-.011	3	.009	15	0	3
77	M15	1	max	-1.826	15	683.147	2	-7.078	15	.013	2	.19	1	0	2
78			min	-37.672	1	-351.603	3	-148.786	1	-.01	3	.009	15	0	3
79		2	max	-1.826	15	492.802	2	-5.49	15	.013	2	.073	1	.271	3
80			min	-37.672	1	-257.089	3	-115.194	1	-.01	3	.004	15	-.523	2
81		3	max	-1.826	15	302.456	2	-3.902	15	.013	2	.004	3	.457	3
82			min	-37.672	1	-162.574	3	-81.603	1	-.01	3	-.015	1	-.876	2
83		4	max	-1.826	15	112.111	2	-2.314	15	.013	2	-.001	12	.56	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-37.672	1	-68.06	3	-48.011	1	-.01	3	-.072	1	-1.06	2
85		5	max	-1.826	15	26.455	3	-.726	15	.013	2	-.004	12	.578	3
86			min	-37.672	1	-78.234	2	-14.42	1	-.01	3	-.1	1	-1.075	2
87		6	max	-1.826	15	120.969	3	19.172	1	.013	2	-.005	15	.513	3
88			min	-37.672	1	-268.58	2	-1.062	3	-.01	3	-.098	1	-.921	2
89		7	max	-1.826	15	215.484	3	52.763	1	.013	2	-.003	15	.363	3
90			min	-37.672	1	-458.925	2	.976	12	-.01	3	-.066	1	-.598	2
91		8	max	-1.826	15	309.998	3	86.355	1	.013	2	.002	10	.129	3
92			min	-37.672	1	-649.271	2	2.564	12	-.01	3	-.006	3	-.105	2
93		9	max	-1.826	15	404.513	3	119.947	1	.013	2	.088	1	.556	2
94			min	-37.672	1	-839.616	2	4.152	12	-.01	3	-.001	3	-.188	3
95		10	max	-1.826	15	1029.962	2	21.109	10	.013	2	.209	1	1.387	2
96			min	-37.672	1	-602.2	10	-153.538	1	-.01	3	.004	12	-.59	3
97		11	max	-1.826	15	839.616	2	-4.152	12	.01	3	.088	1	.556	2
98			min	-37.672	1	-404.513	3	-119.947	1	-.013	2	-.001	3	-.188	3
99		12	max	-1.826	15	649.271	2	-2.564	12	.01	3	.002	10	.129	3
100			min	-37.672	1	-309.998	3	-86.355	1	-.013	2	-.006	3	-.105	2
101		13	max	-1.826	15	458.925	2	-.976	12	.01	3	-.003	15	.363	3
102			min	-37.672	1	-215.484	3	-52.763	1	-.013	2	-.066	1	-.598	2
103		14	max	-1.826	15	268.58	2	1.062	3	.01	3	-.005	15	.513	3
104			min	-37.672	1	-120.969	3	-19.172	1	-.013	2	-.098	1	-.921	2
105		15	max	-1.826	15	78.234	2	14.42	1	.01	3	-.004	12	.578	3
106			min	-37.672	1	-26.455	3	.726	15	-.013	2	-.1	1	-1.075	2
107		16	max	-1.826	15	68.06	3	48.011	1	.01	3	-.001	12	.56	3
108			min	-37.672	1	-112.111	2	2.314	15	-.013	2	-.072	1	-1.06	2
109		17	max	-1.826	15	162.574	3	81.603	1	.01	3	.004	3	.457	3
110			min	-37.672	1	-302.456	2	3.902	15	-.013	2	-.015	1	-.876	2
111		18	max	-1.826	15	257.089	3	115.194	1	.01	3	.073	1	.271	3
112			min	-37.672	1	-492.802	2	5.49	15	-.013	2	.004	15	-.523	2
113		19	max	-1.826	15	351.603	3	148.786	1	.01	3	.19	1	0	2
114			min	-37.672	1	-683.147	2	7.078	15	-.013	2	.009	15	0	3
115	M16	1	max	-3.543	15	636.09	2	-6.835	15	.009	2	.163	1	0	2
116			min	-73.443	1	-310.787	3	-143.806	1	-.013	3	.008	15	0	3
117		2	max	-3.543	15	445.745	2	-5.247	15	.009	2	.051	1	.234	3
118			min	-73.443	1	-216.273	3	-110.215	1	-.013	3	.002	15	-.481	2
119		3	max	-3.543	15	255.399	2	-3.659	15	.009	2	.002	3	.384	3
120			min	-73.443	1	-121.758	3	-76.623	1	-.013	3	-.032	1	-.792	2
121		4	max	-3.543	15	65.054	2	-2.071	15	.009	2	-.003	12	.451	3
122			min	-73.443	1	-27.244	3	-43.032	1	-.013	3	-.086	1	-.935	2
123		5	max	-3.543	15	67.271	3	-.366	10	.009	2	-.005	12	.433	3
124			min	-73.443	1	-125.292	2	-9.44	1	-.013	3	-.109	1	-.908	2
125		6	max	-3.543	15	161.785	3	24.151	1	.009	2	-.005	15	.331	3
126			min	-73.443	1	-315.637	2	-.187	3	-.013	3	-.102	1	-.712	2
127		7	max	-3.543	15	256.3	3	57.743	1	.009	2	-.003	15	.145	3
128			min	-73.443	1	-505.983	2	1.539	12	-.013	3	-.066	1	-.347	2
129		8	max	-3.543	15	350.814	3	91.334	1	.009	2	.003	2	.187	2
130			min	-73.443	1	-696.328	2	3.126	12	-.013	3	-.005	3	-.125	3
131		9	max	-3.543	15	445.329	3	124.926	1	.009	2	.096	1	.891	2
132			min	-73.443	1	-886.674	2	4.714	12	-.013	3	0	3	-.478	3
133		10	max	-3.543	15	1077.019	2	-6.302	12	.009	2	.222	1	1.764	2
134			min	-73.443	1	-539.843	3	-158.518	1	-.013	3	.006	12	-.916	3
135		11	max	-3.543	15	886.674	2	-4.714	12	.013	3	.096	1	.891	2
136			min	-73.443	1	-445.329	3	-124.926	1	-.009	2	0	3	-.478	3
137		12	max	-3.543	15	696.328	2	-3.126	12	.013	3	.003	2	.187	2
138			min	-73.443	1	-350.814	3	-91.334	1	-.009	2	-.005	3	-.125	3
139		13	max	-3.543	15	505.983	2	-1.539	12	.013	3	-.003	15	.145	3
140			min	-73.443	1	-256.3	3	-57.743	1	-.009	2	-.066	1	-.347	2





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-3.543	15	315.637	2	.187	3	.013	3	-.005	15	.331	3
142			min	-73.443	1	-161.785	3	-24.151	1	-.009	2	-.102	1	-.712	2
143		15	max	-3.543	15	125.292	2	9.44	1	.013	3	-.005	12	.433	3
144			min	-73.443	1	-67.271	3	.366	10	-.009	2	-.109	1	-.908	2
145		16	max	-3.543	15	27.244	3	43.032	1	.013	3	-.003	12	.451	3
146			min	-73.443	1	-65.054	2	2.071	15	-.009	2	-.086	1	-.935	2
147		17	max	-3.543	15	121.758	3	76.623	1	.013	3	.002	3	.384	3
148			min	-73.443	1	-255.399	2	3.659	15	-.009	2	-.032	1	-.792	2
149		18	max	-3.543	15	216.273	3	110.215	1	.013	3	.051	1	.234	3
150			min	-73.443	1	-445.745	2	5.247	15	-.009	2	.002	15	-.481	2
151		19	max	-3.543	15	310.787	3	143.806	1	.013	3	.163	1	0	2
152			min	-73.443	1	-636.09	2	6.835	15	-.009	2	.008	15	0	3
153	M2	1	max	1030.535	2	1.931	4	.314	1	0	3	0	3	0	1
154			min	-1458.384	3	.454	15	.015	15	0	1	0	2	0	1
155		2	max	1031.01	2	1.845	4	.314	1	0	3	0	1	0	15
156			min	-1458.028	3	.434	15	.015	15	0	1	0	15	0	4
157		3	max	1031.486	2	1.76	4	.314	1	0	3	0	1	0	15
158			min	-1457.671	3	.414	15	.015	15	0	1	0	15	-.001	4
159		4	max	1031.962	2	1.674	4	.314	1	0	3	0	1	0	15
160			min	-1457.314	3	.394	15	.015	15	0	1	0	15	-.002	4
161		5	max	1032.438	2	1.588	4	.314	1	0	3	0	1	0	15
162			min	-1456.957	3	.374	15	.015	15	0	1	0	15	-.002	4
163		6	max	1032.913	2	1.503	4	.314	1	0	3	0	1	0	15
164			min	-1456.6	3	.354	15	.015	15	0	1	0	15	-.003	4
165		7	max	1033.389	2	1.417	4	.314	1	0	3	0	1	0	15
166			min	-1456.244	3	.334	15	.015	15	0	1	0	15	-.003	4
167		8	max	1033.865	2	1.332	4	.314	1	0	3	0	1	0	15
168			min	-1455.887	3	.313	15	.015	15	0	1	0	15	-.004	4
169		9	max	1034.341	2	1.246	4	.314	1	0	3	0	1	0	15
170			min	-1455.53	3	.28	12	.015	15	0	1	0	15	-.004	4
171		10	max	1034.816	2	1.161	4	.314	1	0	3	0	1	-.001	15
172			min	-1455.173	3	.247	12	.015	15	0	1	0	15	-.005	4
173		11	max	1035.292	2	1.08	2	.314	1	0	3	.001	1	-.001	15
174			min	-1454.816	3	.214	12	.015	15	0	1	0	15	-.005	4
175		12	max	1035.768	2	1.013	2	.314	1	0	3	.001	1	-.001	15
176			min	-1454.459	3	.18	12	.015	15	0	1	0	15	-.005	4
177		13	max	1036.244	2	.947	2	.314	1	0	3	.001	1	-.001	15
178			min	-1454.103	3	.147	12	.015	15	0	1	0	15	-.006	4
179		14	max	1036.719	2	.88	2	.314	1	0	3	.001	1	-.001	15
180			min	-1453.746	3	.114	12	.015	15	0	1	0	15	-.006	4
181		15	max	1037.195	2	.813	2	.314	1	0	3	.001	1	-.001	15
182			min	-1453.389	3	.08	12	.015	15	0	1	0	15	-.006	4
183		16	max	1037.671	2	.747	2	.314	1	0	3	.002	1	-.001	12
184			min	-1453.032	3	.047	12	.015	15	0	1	0	15	-.006	4
185		17	max	1038.147	2	.68	2	.314	1	0	3	.002	1	-.001	12
186			min	-1452.675	3	0	3	.015	15	0	1	0	15	-.006	4
187		18	max	1038.622	2	.613	2	.314	1	0	3	.002	1	-.001	12
188			min	-1452.319	3	-.05	3	.015	15	0	1	0	15	-.007	4
189		19	max	1039.098	2	.546	2	.314	1	0	3	.002	1	-.001	12
190			min	-1451.962	3	-.1	3	.015	15	0	1	0	15	-.007	4
191	M3	1	max	786.074	2	7.78	4	.169	1	0	3	0	1	.007	4
192			min	-906.219	3	1.829	15	.008	15	0	1	0	15	.001	12
193		2	max	785.904	2	7.015	4	.169	1	0	3	0	1	.004	2
194			min	-906.347	3	1.649	15	.008	15	0	1	0	15	0	12
195		3	max	785.734	2	6.251	4	.169	1	0	3	0	1	.002	2
196			min	-906.475	3	1.47	15	.008	15	0	1	0	15	-.001	3
197		4	max	785.563	2	5.487	4	.169	1	0	3	0	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-906.603	3	1.29	15	.008	15	0	1	0	15	-.002	3
199		5	max	785.393	2	4.722	4	.169	1	0	3	0	1	0	15
200			min	-906.731	3	1.11	15	.008	15	0	1	0	15	-.004	4
201		6	max	785.222	2	3.958	4	.169	1	0	3	0	1	-.001	15
202			min	-906.858	3	.931	15	.008	15	0	1	0	15	-.005	4
203		7	max	785.052	2	3.193	4	.169	1	0	3	0	1	-.002	15
204			min	-906.986	3	.751	15	.008	15	0	1	0	15	-.007	4
205		8	max	784.882	2	2.429	4	.169	1	0	3	0	1	-.002	15
206			min	-907.114	3	.571	15	.008	15	0	1	0	15	-.008	4
207		9	max	784.711	2	1.664	4	.169	1	0	3	0	1	-.002	15
208			min	-907.242	3	.392	15	.008	15	0	1	0	15	-.009	4
209		10	max	784.541	2	.9	4	.169	1	0	3	0	1	-.002	15
210			min	-907.369	3	.192	12	.008	15	0	1	0	15	-.01	4
211		11	max	784.371	2	.293	2	.169	1	0	3	0	1	-.002	15
212			min	-907.497	3	-.175	3	.008	15	0	1	0	15	-.01	4
213		12	max	784.2	2	-.148	15	.169	1	0	3	.001	1	-.002	15
214			min	-907.625	3	-.629	4	.008	15	0	1	0	15	-.01	4
215		13	max	784.03	2	-.327	15	.169	1	0	3	.001	1	-.002	15
216			min	-907.753	3	-1.393	4	.008	15	0	1	0	15	-.009	4
217		14	max	783.86	2	-.507	15	.169	1	0	3	.001	1	-.002	15
218			min	-907.88	3	-2.158	4	.008	15	0	1	0	15	-.008	4
219		15	max	783.689	2	-.687	15	.169	1	0	3	.001	1	-.002	15
220			min	-908.008	3	-2.922	4	.008	15	0	1	0	15	-.007	4
221		16	max	783.519	2	-.866	15	.169	1	0	3	.001	1	-.001	15
222			min	-908.136	3	-3.687	4	.008	15	0	1	0	15	-.006	4
223		17	max	783.349	2	-1.046	15	.169	1	0	3	.001	1	-.001	15
224			min	-908.264	3	-4.451	4	.008	15	0	1	0	15	-.004	4
225		18	max	783.178	2	-1.226	15	.169	1	0	3	.001	1	0	15
226			min	-908.391	3	-5.216	4	.008	15	0	1	0	15	-.002	4
227		19	max	783.008	2	-1.405	15	.169	1	0	3	.002	1	0	1
228			min	-908.519	3	-5.98	4	.008	15	0	1	0	15	0	1
229	M4	1	max	906.897	1	0	1	-.356	15	0	1	.001	1	0	1
230			min	-41.917	3	0	1	-7.401	1	0	1	0	15	0	1
231		2	max	907.067	1	0	1	-.356	15	0	1	0	1	0	1
232			min	-41.789	3	0	1	-7.401	1	0	1	0	15	0	1
233		3	max	907.238	1	0	1	-.356	15	0	1	0	15	0	1
234			min	-41.662	3	0	1	-7.401	1	0	1	0	1	0	1
235		4	max	907.408	1	0	1	-.356	15	0	1	0	15	0	1
236			min	-41.534	3	0	1	-7.401	1	0	1	-.001	1	0	1
237		5	max	907.578	1	0	1	-.356	15	0	1	0	15	0	1
238			min	-41.406	3	0	1	-7.401	1	0	1	-.002	1	0	1
239		6	max	907.749	1	0	1	-.356	15	0	1	0	15	0	1
240			min	-41.278	3	0	1	-7.401	1	0	1	-.003	1	0	1
241		7	max	907.919	1	0	1	-.356	15	0	1	0	15	0	1
242			min	-41.151	3	0	1	-7.401	1	0	1	-.004	1	0	1
243		8	max	908.089	1	0	1	-.356	15	0	1	0	15	0	1
244			min	-41.023	3	0	1	-7.401	1	0	1	-.005	1	0	1
245		9	max	908.26	1	0	1	-.356	15	0	1	0	15	0	1
246			min	-40.895	3	0	1	-7.401	1	0	1	-.006	1	0	1
247		10	max	908.43	1	0	1	-.356	15	0	1	0	15	0	1
248			min	-40.767	3	0	1	-7.401	1	0	1	-.006	1	0	1
249		11	max	908.6	1	0	1	-.356	15	0	1	0	15	0	1
250			min	-40.64	3	0	1	-7.401	1	0	1	-.007	1	0	1
251		12	max	908.771	1	0	1	-.356	15	0	1	0	15	0	1
252			min	-40.512	3	0	1	-7.401	1	0	1	-.008	1	0	1
253		13	max	908.941	1	0	1	-.356	15	0	1	0	15	0	1
254			min	-40.384	3	0	1	-7.401	1	0	1	-.009	1	0	1





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	909.112	1	0	1	-356	15	0	1	0	15	0	1
256		min	-40.256	3	0	1	-7.401	1	0	1	-.01	1	0	1
257	15	max	909.282	1	0	1	-356	15	0	1	0	15	0	1
258		min	-40.128	3	0	1	-7.401	1	0	1	-.011	1	0	1
259	16	max	909.452	1	0	1	-356	15	0	1	0	15	0	1
260		min	-40.001	3	0	1	-7.401	1	0	1	-.011	1	0	1
261	17	max	909.623	1	0	1	-356	15	0	1	0	15	0	1
262		min	-39.873	3	0	1	-7.401	1	0	1	-.012	1	0	1
263	18	max	909.793	1	0	1	-356	15	0	1	0	15	0	1
264		min	-39.745	3	0	1	-7.401	1	0	1	-.013	1	0	1
265	19	max	909.963	1	0	1	-356	15	0	1	0	15	0	1
266		min	-39.617	3	0	1	-7.401	1	0	1	-.014	1	0	1
267	M6	1	max	3229.102	2	2.313	2	0	1	0	0	1	0	1
268		min	-4668.35	3	.098	3	0	1	0	1	0	1	0	1
269	2	max	3229.578	2	2.247	2	0	1	0	1	0	1	0	3
270		min	-4667.993	3	.048	3	0	1	0	1	0	1	0	2
271	3	max	3230.054	2	2.18	2	0	1	0	1	0	1	0	3
272		min	-4667.636	3	-.002	3	0	1	0	1	0	1	-.001	2
273	4	max	3230.529	2	2.113	2	0	1	0	1	0	1	0	3
274		min	-4667.279	3	-.052	3	0	1	0	1	0	1	-.002	2
275	5	max	3231.005	2	2.047	2	0	1	0	1	0	1	0	3
276		min	-4666.922	3	-.102	3	0	1	0	1	0	1	-.003	2
277	6	max	3231.481	2	1.98	2	0	1	0	1	0	1	0	3
278		min	-4666.566	3	-.152	3	0	1	0	1	0	1	-.003	2
279	7	max	3231.957	2	1.913	2	0	1	0	1	0	1	0	3
280		min	-4666.209	3	-.202	3	0	1	0	1	0	1	-.004	2
281	8	max	3232.432	2	1.846	2	0	1	0	1	0	1	0	3
282		min	-4665.852	3	-.252	3	0	1	0	1	0	1	-.005	2
283	9	max	3232.908	2	1.78	2	0	1	0	1	0	1	0	3
284		min	-4665.495	3	-.302	3	0	1	0	1	0	1	-.005	2
285	10	max	3233.384	2	1.713	2	0	1	0	1	0	1	0	3
286		min	-4665.138	3	-.352	3	0	1	0	1	0	1	-.006	2
287	11	max	3233.86	2	1.646	2	0	1	0	1	0	1	0	3
288		min	-4664.781	3	-.402	3	0	1	0	1	0	1	-.006	2
289	12	max	3234.335	2	1.58	2	0	1	0	1	0	1	0	3
290		min	-4664.425	3	-.452	3	0	1	0	1	0	1	-.007	2
291	13	max	3234.811	2	1.513	2	0	1	0	1	0	1	0	3
292		min	-4664.068	3	-.502	3	0	1	0	1	0	1	-.007	2
293	14	max	3235.287	2	1.446	2	0	1	0	1	0	1	0	3
294		min	-4663.711	3	-.552	3	0	1	0	1	0	1	-.008	2
295	15	max	3235.763	2	1.38	2	0	1	0	1	0	1	.001	3
296		min	-4663.354	3	-.602	3	0	1	0	1	0	1	-.008	2
297	16	max	3236.238	2	1.313	2	0	1	0	1	0	1	.001	3
298		min	-4662.997	3	-.652	3	0	1	0	1	0	1	-.009	2
299	17	max	3236.714	2	1.246	2	0	1	0	1	0	1	.002	3
300		min	-4662.641	3	-.702	3	0	1	0	1	0	1	-.009	2
301	18	max	3237.19	2	1.18	2	0	1	0	1	0	1	.002	3
302		min	-4662.284	3	-.752	3	0	1	0	1	0	1	-.01	2
303	19	max	3237.666	2	1.113	2	0	1	0	1	0	1	.002	3
304		min	-4661.927	3	-.802	3	0	1	0	1	0	1	-.01	2
305	M7	1	max	2704.543	2	7.804	4	0	1	0	0	1	.01	2
306		min	-2794.498	3	1.833	15	0	1	0	1	0	1	-.002	3
307	2	max	2704.372	2	7.04	4	0	1	0	1	0	1	.007	2
308		min	-2794.626	3	1.653	15	0	1	0	1	0	1	-.004	3
309	3	max	2704.202	2	6.275	4	0	1	0	1	0	1	.005	2
310		min	-2794.754	3	1.473	15	0	1	0	1	0	1	-.005	3
311	4	max	2704.032	2	5.511	4	0	1	0	1	0	1	.003	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2794.882	3	1.294	15	0	1	0	1	0	1	-.006	3
313	5	max	2703.861	2	4.746	4	0	1	0	1	0	1	0	2
314		min	-2795.009	3	1.114	15	0	1	0	1	0	1	-.007	3
315	6	max	2703.691	2	3.982	4	0	1	0	1	0	1	0	2
316		min	-2795.137	3	.934	15	0	1	0	1	0	1	-.008	3
317	7	max	2703.521	2	3.218	4	0	1	0	1	0	1	-.002	15
318		min	-2795.265	3	.745	12	0	1	0	1	0	1	-.008	3
319	8	max	2703.35	2	2.52	2	0	1	0	1	0	1	-.002	15
320		min	-2795.393	3	.447	12	0	1	0	1	0	1	-.008	3
321	9	max	2703.18	2	1.924	2	0	1	0	1	0	1	-.002	15
322		min	-2795.52	3	1.149	12	0	1	0	1	0	1	-.009	4
323	10	max	2703.01	2	1.328	2	0	1	0	1	0	1	-.002	15
324		min	-2795.648	3	-.273	3	0	1	0	1	0	1	-.009	4
325	11	max	2702.839	2	.733	2	0	1	0	1	0	1	-.002	15
326		min	-2795.776	3	-.72	3	0	1	0	1	0	1	-.01	4
327	12	max	2702.669	2	.137	2	0	1	0	1	0	1	-.002	15
328		min	-2795.904	3	-1.167	3	0	1	0	1	0	1	-.01	4
329	13	max	2702.499	2	-.323	15	0	1	0	1	0	1	-.002	15
330		min	-2796.031	3	-1.613	3	0	1	0	1	0	1	-.009	4
331	14	max	2702.328	2	-.503	15	0	1	0	1	0	1	-.002	15
332		min	-2796.159	3	-2.134	4	0	1	0	1	0	1	-.008	4
333	15	max	2702.158	2	-.683	15	0	1	0	1	0	1	-.002	15
334		min	-2796.287	3	-2.898	4	0	1	0	1	0	1	-.007	4
335	16	max	2701.987	2	-.863	15	0	1	0	1	0	1	-.001	15
336		min	-2796.415	3	-3.662	4	0	1	0	1	0	1	-.006	4
337	17	max	2701.817	2	-1.042	15	0	1	0	1	0	1	-.001	15
338		min	-2796.542	3	-4.427	4	0	1	0	1	0	1	-.004	4
339	18	max	2701.647	2	-1.222	15	0	1	0	1	0	1	0	15
340		min	-2796.67	3	-5.191	4	0	1	0	1	0	1	-.002	4
341	19	max	2701.476	2	-1.402	15	0	1	0	1	0	1	0	1
342		min	-2796.798	3	-5.956	4	0	1	0	1	0	1	0	1
343	M8	1	max	2436.874	2	0	1	0	1	0	1	0	1	1
344		min	-229.524	3	0	1	0	1	0	1	0	1	0	1
345	2	max	2437.044	2	0	1	0	1	0	1	0	1	0	1
346		min	-229.396	3	0	1	0	1	0	1	0	1	0	1
347	3	max	2437.215	2	0	1	0	1	0	1	0	1	0	1
348		min	-229.268	3	0	1	0	1	0	1	0	1	0	1
349	4	max	2437.385	2	0	1	0	1	0	1	0	1	0	1
350		min	-229.14	3	0	1	0	1	0	1	0	1	0	1
351	5	max	2437.556	2	0	1	0	1	0	1	0	1	0	1
352		min	-229.013	3	0	1	0	1	0	1	0	1	0	1
353	6	max	2437.726	2	0	1	0	1	0	1	0	1	0	1
354		min	-228.885	3	0	1	0	1	0	1	0	1	0	1
355	7	max	2437.896	2	0	1	0	1	0	1	0	1	0	1
356		min	-228.757	3	0	1	0	1	0	1	0	1	0	1
357	8	max	2438.067	2	0	1	0	1	0	1	0	1	0	1
358		min	-228.629	3	0	1	0	1	0	1	0	1	0	1
359	9	max	2438.237	2	0	1	0	1	0	1	0	1	0	1
360		min	-228.502	3	0	1	0	1	0	1	0	1	0	1
361	10	max	2438.407	2	0	1	0	1	0	1	0	1	0	1
362		min	-228.374	3	0	1	0	1	0	1	0	1	0	1
363	11	max	2438.578	2	0	1	0	1	0	1	0	1	0	1
364		min	-228.246	3	0	1	0	1	0	1	0	1	0	1
365	12	max	2438.748	2	0	1	0	1	0	1	0	1	0	1
366		min	-228.118	3	0	1	0	1	0	1	0	1	0	1
367	13	max	2438.918	2	0	1	0	1	0	1	0	1	0	1
368		min	-227.991	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2439.089	2	0	1	0	1	0	1	0	1	0	1
370			min	-227.863	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2439.259	2	0	1	0	1	0	1	0	1	0	1
372			min	-227.735	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2439.429	2	0	1	0	1	0	1	0	1	0	1
374			min	-227.607	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2439.6	2	0	1	0	1	0	1	0	1	0	1
376			min	-227.48	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2439.77	2	0	1	0	1	0	1	0	1	0	1
378			min	-227.352	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2439.94	2	0	1	0	1	0	1	0	1	0	1
380			min	-227.224	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1030.535	2	1.931	4	-.015	15	0	1	0	2	0	1
382			min	-1458.384	3	.454	15	-.314	1	0	3	0	3	0	1
383		2	max	1031.01	2	1.845	4	-.015	15	0	1	0	15	0	15
384			min	-1458.028	3	.434	15	-.314	1	0	3	0	1	0	4
385		3	max	1031.486	2	1.76	4	-.015	15	0	1	0	15	0	15
386			min	-1457.671	3	.414	15	-.314	1	0	3	0	1	-.001	4
387		4	max	1031.962	2	1.674	4	-.015	15	0	1	0	15	0	15
388			min	-1457.314	3	.394	15	-.314	1	0	3	0	1	-.002	4
389		5	max	1032.438	2	1.588	4	-.015	15	0	1	0	15	0	15
390			min	-1456.957	3	.374	15	-.314	1	0	3	0	1	-.002	4
391		6	max	1032.913	2	1.503	4	-.015	15	0	1	0	15	0	15
392			min	-1456.6	3	.354	15	-.314	1	0	3	0	1	-.003	4
393		7	max	1033.389	2	1.417	4	-.015	15	0	1	0	15	0	15
394			min	-1456.244	3	.334	15	-.314	1	0	3	0	1	-.003	4
395		8	max	1033.865	2	1.332	4	-.015	15	0	1	0	15	0	15
396			min	-1455.887	3	.313	15	-.314	1	0	3	0	1	-.004	4
397		9	max	1034.341	2	1.246	4	-.015	15	0	1	0	15	0	15
398			min	-1455.53	3	.28	12	-.314	1	0	3	0	1	-.004	4
399		10	max	1034.816	2	1.161	4	-.015	15	0	1	0	15	-.001	15
400			min	-1455.173	3	.247	12	-.314	1	0	3	0	1	-.005	4
401		11	max	1035.292	2	1.08	2	-.015	15	0	1	0	15	-.001	15
402			min	-1454.816	3	.214	12	-.314	1	0	3	-.001	1	-.005	4
403		12	max	1035.768	2	1.013	2	-.015	15	0	1	0	15	-.001	15
404			min	-1454.459	3	.18	12	-.314	1	0	3	-.001	1	-.005	4
405		13	max	1036.244	2	.947	2	-.015	15	0	1	0	15	-.001	15
406			min	-1454.103	3	.147	12	-.314	1	0	3	-.001	1	-.006	4
407		14	max	1036.719	2	.88	2	-.015	15	0	1	0	15	-.001	15
408			min	-1453.746	3	.114	12	-.314	1	0	3	-.001	1	-.006	4
409		15	max	1037.195	2	.813	2	-.015	15	0	1	0	15	-.001	15
410			min	-1453.389	3	.08	12	-.314	1	0	3	-.001	1	-.006	4
411		16	max	1037.671	2	.747	2	-.015	15	0	1	0	15	-.001	12
412			min	-1453.032	3	.047	12	-.314	1	0	3	-.002	1	-.006	4
413		17	max	1038.147	2	.68	2	-.015	15	0	1	0	15	-.001	12
414			min	-1452.675	3	0	3	-.314	1	0	3	-.002	1	-.006	4
415		18	max	1038.622	2	.613	2	-.015	15	0	1	0	15	-.001	12
416			min	-1452.319	3	-.05	3	-.314	1	0	3	-.002	1	-.007	4
417		19	max	1039.098	2	.546	2	-.015	15	0	1	0	15	-.001	12
418			min	-1451.962	3	-.1	3	-.314	1	0	3	-.002	1	-.007	4
419	M11	1	max	786.074	2	7.78	4	-.008	15	0	1	0	15	.007	4
420			min	-906.219	3	1.829	15	-.169	1	0	3	0	1	.001	12
421		2	max	785.904	2	7.015	4	-.008	15	0	1	0	15	.004	2
422			min	-906.347	3	1.649	15	-.169	1	0	3	0	1	0	12
423		3	max	785.734	2	6.251	4	-.008	15	0	1	0	15	.002	2
424			min	-906.475	3	1.47	15	-.169	1	0	3	0	1	-.001	3
425		4	max	785.563	2	5.487	4	-.008	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-906.603	3	1.29	15	-1.169	1	0	3	0	1	-0.002	3
427		5	max	785.393	2	4.722	4	-0.008	15	0	1	0	15	0	15
428			min	-906.731	3	1.11	15	-1.169	1	0	3	0	1	-0.004	4
429		6	max	785.222	2	3.958	4	-0.008	15	0	1	0	15	-0.001	15
430			min	-906.858	3	.931	15	-1.169	1	0	3	0	1	-0.005	4
431		7	max	785.052	2	3.193	4	-0.008	15	0	1	0	15	-0.002	15
432			min	-906.986	3	.751	15	-1.169	1	0	3	0	1	-0.007	4
433		8	max	784.882	2	2.429	4	-0.008	15	0	1	0	15	-0.002	15
434			min	-907.114	3	.571	15	-1.169	1	0	3	0	1	-0.008	4
435		9	max	784.711	2	1.664	4	-0.008	15	0	1	0	15	-0.002	15
436			min	-907.242	3	.392	15	-1.169	1	0	3	0	1	-0.009	4
437		10	max	784.541	2	.9	4	-0.008	15	0	1	0	15	-0.002	15
438			min	-907.369	3	.192	12	-1.169	1	0	3	0	1	-.01	4
439		11	max	784.371	2	.293	2	-0.008	15	0	1	0	15	-0.002	15
440			min	-907.497	3	-.175	3	-1.169	1	0	3	0	1	-.01	4
441		12	max	784.2	2	-.148	15	-0.008	15	0	1	0	15	-0.002	15
442			min	-907.625	3	-.629	4	-1.169	1	0	3	-.001	1	-.01	4
443		13	max	784.03	2	-.327	15	-0.008	15	0	1	0	15	-0.002	15
444			min	-907.753	3	-1.393	4	-1.169	1	0	3	-.001	1	-0.009	4
445		14	max	783.86	2	-.507	15	-0.008	15	0	1	0	15	-0.002	15
446			min	-907.88	3	-2.158	4	-1.169	1	0	3	-.001	1	-0.008	4
447		15	max	783.689	2	-.687	15	-0.008	15	0	1	0	15	-0.002	15
448			min	-908.008	3	-2.922	4	-1.169	1	0	3	-.001	1	-0.007	4
449		16	max	783.519	2	-.866	15	-0.008	15	0	1	0	15	-0.001	15
450			min	-908.136	3	-3.687	4	-1.169	1	0	3	-.001	1	-0.006	4
451		17	max	783.349	2	-1.046	15	-0.008	15	0	1	0	15	-0.001	15
452			min	-908.264	3	-4.451	4	-1.169	1	0	3	-.001	1	-0.004	4
453		18	max	783.178	2	-1.226	15	-0.008	15	0	1	0	15	0	15
454			min	-908.391	3	-5.216	4	-1.169	1	0	3	-.001	1	-0.002	4
455		19	max	783.008	2	-1.405	15	-0.008	15	0	1	0	15	0	1
456			min	-908.519	3	-5.98	4	-1.169	1	0	3	-.002	1	0	1
457	M12	1	max	906.897	1	0	1	7.401	1	0	1	0	15	0	1
458			min	-41.917	3	0	1	.356	15	0	1	-.001	1	0	1
459		2	max	907.067	1	0	1	7.401	1	0	1	0	15	0	1
460			min	-41.789	3	0	1	.356	15	0	1	0	1	0	1
461		3	max	907.238	1	0	1	7.401	1	0	1	0	1	0	1
462			min	-41.662	3	0	1	.356	15	0	1	0	15	0	1
463		4	max	907.408	1	0	1	7.401	1	0	1	.001	1	0	1
464			min	-41.534	3	0	1	.356	15	0	1	0	15	0	1
465		5	max	907.578	1	0	1	7.401	1	0	1	.002	1	0	1
466			min	-41.406	3	0	1	.356	15	0	1	0	15	0	1
467		6	max	907.749	1	0	1	7.401	1	0	1	.003	1	0	1
468			min	-41.278	3	0	1	.356	15	0	1	0	15	0	1
469		7	max	907.919	1	0	1	7.401	1	0	1	.004	1	0	1
470			min	-41.151	3	0	1	.356	15	0	1	0	15	0	1
471		8	max	908.089	1	0	1	7.401	1	0	1	.005	1	0	1
472			min	-41.023	3	0	1	.356	15	0	1	0	15	0	1
473		9	max	908.26	1	0	1	7.401	1	0	1	.006	1	0	1
474			min	-40.895	3	0	1	.356	15	0	1	0	15	0	1
475		10	max	908.43	1	0	1	7.401	1	0	1	.006	1	0	1
476			min	-40.767	3	0	1	.356	15	0	1	0	15	0	1
477		11	max	908.6	1	0	1	7.401	1	0	1	.007	1	0	1
478			min	-40.64	3	0	1	.356	15	0	1	0	15	0	1
479		12	max	908.771	1	0	1	7.401	1	0	1	.008	1	0	1
480			min	-40.512	3	0	1	.356	15	0	1	0	15	0	1
481		13	max	908.941	1	0	1	7.401	1	0	1	.009	1	0	1
482			min	-40.384	3	0	1	.356	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	909.112	1	0	1	7.401	1	0	1	.01	1	0	1
484			min	-40.256	3	0	1	.356	15	0	1	0	15	0	1
485		15	max	909.282	1	0	1	7.401	1	0	1	.011	1	0	1
486			min	-40.128	3	0	1	.356	15	0	1	0	15	0	1
487		16	max	909.452	1	0	1	7.401	1	0	1	.011	1	0	1
488			min	-40.001	3	0	1	.356	15	0	1	0	15	0	1
489		17	max	909.623	1	0	1	7.401	1	0	1	.012	1	0	1
490			min	-39.873	3	0	1	.356	15	0	1	0	15	0	1
491		18	max	909.793	1	0	1	7.401	1	0	1	.013	1	0	1
492			min	-39.745	3	0	1	.356	15	0	1	0	15	0	1
493		19	max	909.963	1	0	1	7.401	1	0	1	.014	1	0	1
494			min	-39.617	3	0	1	.356	15	0	1	0	15	0	1
495	M1	1	max	143.459	1	774.498	3	-3.287	15	0	2	.162	1	0	15
496			min	6.827	15	-421.514	2	-67.95	1	0	3	.008	15	-.014	2
497		2	max	144.175	1	773.567	3	-3.287	15	0	2	.126	1	.208	2
498			min	7.043	15	-422.755	2	-67.95	1	0	3	.006	15	-.409	3
499		3	max	561.114	3	527.948	2	-3.272	15	0	3	.09	1	.42	2
500			min	-324.692	2	-583.231	3	-67.74	1	0	2	.004	15	-.801	3
501		4	max	561.651	3	526.708	2	-3.272	15	0	3	.054	1	.142	2
502			min	-323.976	2	-584.161	3	-67.74	1	0	2	.003	15	-.493	3
503		5	max	562.188	3	525.467	2	-3.272	15	0	3	.019	1	-.003	15
504			min	-323.26	2	-585.091	3	-67.74	1	0	2	0	15	-.184	3
505		6	max	562.725	3	524.227	2	-3.272	15	0	3	0	15	.125	3
506			min	-322.544	2	-586.022	3	-67.74	1	0	2	-.017	1	-.412	2
507		7	max	563.263	3	522.986	2	-3.272	15	0	3	-.003	15	.434	3
508			min	-321.828	2	-586.952	3	-67.74	1	0	2	-.053	1	-.689	2
509		8	max	563.8	3	521.746	2	-3.272	15	0	3	-.004	15	.744	3
510			min	-321.111	2	-587.882	3	-67.74	1	0	2	-.089	1	-.964	2
511		9	max	577.511	3	52.771	2	-5.022	15	0	9	.054	1	.867	3
512			min	-259.457	2	.378	15	-104.084	1	0	3	.003	15	-1.103	2
513		10	max	578.048	3	51.53	2	-5.022	15	0	9	0	10	.847	3
514			min	-258.74	2	.004	15	-104.084	1	0	3	0	1	-1.131	2
515		11	max	578.585	3	50.29	2	-5.022	15	0	9	-.003	15	.827	3
516			min	-258.024	2	-1.529	4	-104.084	1	0	3	-.056	1	-1.158	2
517		12	max	592.093	3	392.306	3	-3.196	15	0	2	.088	1	.723	3
518			min	-196.281	2	-632.113	2	-66.431	1	0	3	.004	15	-1.028	2
519		13	max	592.63	3	391.375	3	-3.196	15	0	2	.053	1	.516	3
520			min	-195.565	2	-633.354	2	-66.431	1	0	3	.003	15	-.694	2
521		14	max	593.167	3	390.445	3	-3.196	15	0	2	.018	1	.31	3
522			min	-194.848	2	-634.594	2	-66.431	1	0	3	0	15	-.359	2
523		15	max	593.705	3	389.515	3	-3.196	15	0	2	0	15	.104	3
524			min	-194.132	2	-635.835	2	-66.431	1	0	3	-.018	1	-.04	1
525		16	max	594.242	3	388.584	3	-3.196	15	0	2	-.003	15	.312	2
526			min	-193.416	2	-637.075	2	-66.431	1	0	3	-.053	1	-.101	3
527		17	max	594.779	3	387.654	3	-3.196	15	0	2	-.004	15	.648	2
528			min	-192.7	2	-638.316	2	-66.431	1	0	3	-.088	1	-.306	3
529		18	max	-7.051	15	637.827	2	-3.543	15	0	3	-.006	15	.327	2
530			min	-144.518	1	-309.941	3	-73.508	1	0	2	-.125	1	-.151	3
531		19	max	-6.835	15	636.586	2	-3.543	15	0	3	-.008	15	.013	3
532			min	-143.802	1	-310.872	3	-73.508	1	0	2	-.163	1	-.009	2
533	M5	1	max	317.73	1	2573.269	3	0	1	0	1	0	1	.029	2
534			min	11.748	12	-1449.158	2	0	1	0	1	0	1	0	15
535		2	max	318.446	1	2572.339	3	0	1	0	1	0	1	.794	2
536			min	12.106	12	-1450.399	2	0	1	0	1	0	1	-1.356	3
537		3	max	1766.026	3	1511.575	2	0	1	0	1	0	1	1.524	2
538			min	-1071.908	2	-1790.972	3	0	1	0	1	0	1	-2.661	3
539		4	max	1766.563	3	1510.334	2	0	1	0	1	0	1	.726	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1071.192	2	-1791.902	3	0	1	0	1	0	1	-1.716	3
541		5	max	1767.1	3	1509.093	2	0	1	0	1	0	1	.015	9
542			min	-1070.476	2	-1792.833	3	0	1	0	1	0	1	-.77	3
543		6	max	1767.637	3	1507.853	2	0	1	0	1	0	1	.176	3
544			min	-1069.759	2	-1793.763	3	0	1	0	1	0	1	-.866	2
545		7	max	1768.174	3	1506.612	2	0	1	0	1	0	1	1.123	3
546			min	-1069.043	2	-1794.694	3	0	1	0	1	0	1	-1.662	2
547		8	max	1768.712	3	1505.372	2	0	1	0	1	0	1	2.07	3
548			min	-1068.327	2	-1795.624	3	0	1	0	1	0	1	-2.456	2
549		9	max	1785.771	3	177.907	2	0	1	0	1	0	1	2.381	3
550			min	-935.392	2	.372	15	0	1	0	1	0	1	-2.805	2
551		10	max	1786.308	3	176.666	2	0	1	0	1	0	1	2.306	3
552			min	-934.676	2	-.002	15	0	1	0	1	0	1	-2.898	2
553		11	max	1786.845	3	175.426	2	0	1	0	1	0	1	2.231	3
554			min	-933.96	2	-1.472	4	0	1	0	1	0	1	-2.991	2
555		12	max	1804.31	3	1175.648	3	0	1	0	1	0	1	1.959	3
556			min	-801.203	2	-1864.618	2	0	1	0	1	0	1	-2.68	2
557		13	max	1804.847	3	1174.718	3	0	1	0	1	0	1	1.338	3
558			min	-800.486	2	-1865.859	2	0	1	0	1	0	1	-1.696	2
559		14	max	1805.384	3	1173.787	3	0	1	0	1	0	1	.719	3
560			min	-799.77	2	-1867.099	2	0	1	0	1	0	1	-.711	2
561		15	max	1805.922	3	1172.857	3	0	1	0	1	0	1	.274	2
562			min	-799.054	2	-1868.34	2	0	1	0	1	0	1	-.002	13
563		16	max	1806.459	3	1171.926	3	0	1	0	1	0	1	1.26	2
564			min	-798.338	2	-1869.58	2	0	1	0	1	0	1	-.519	3
565		17	max	1806.996	3	1170.996	3	0	1	0	1	0	1	2.247	2
566			min	-797.622	2	-1870.821	2	0	1	0	1	0	1	-1.137	3
567		18	max	-12.961	12	2157.628	2	0	1	0	1	0	1	1.157	2
568			min	-317.76	1	-1079.108	3	0	1	0	1	0	1	-.595	3
569		19	max	-12.603	12	2156.388	2	0	1	0	1	0	1	.019	2
570			min	-317.044	1	-1080.039	3	0	1	0	1	0	1	-.025	3
571	M9	1	max	143.459	1	774.498	3	67.95	1	0	3	-.008	15	0	15
572			min	6.827	15	-421.514	2	3.287	15	0	2	-.162	1	-.014	2
573		2	max	144.175	1	773.567	3	67.95	1	0	3	-.006	15	.208	2
574			min	7.043	15	-422.755	2	3.287	15	0	2	-.126	1	-.409	3
575		3	max	561.114	3	527.948	2	67.74	1	0	2	-.004	15	.42	2
576			min	-324.692	2	-583.231	3	3.272	15	0	3	-.09	1	-.801	3
577		4	max	561.651	3	526.708	2	67.74	1	0	2	-.003	15	.142	2
578			min	-323.976	2	-584.161	3	3.272	15	0	3	-.054	1	-.493	3
579		5	max	562.188	3	525.467	2	67.74	1	0	2	0	15	-.003	15
580			min	-323.26	2	-585.091	3	3.272	15	0	3	-.019	1	-.184	3
581		6	max	562.725	3	524.227	2	67.74	1	0	2	.017	1	.125	3
582			min	-322.544	2	-586.022	3	3.272	15	0	3	0	15	-.412	2
583		7	max	563.263	3	522.986	2	67.74	1	0	2	.053	1	.434	3
584			min	-321.828	2	-586.952	3	3.272	15	0	3	.003	15	-.689	2
585		8	max	563.8	3	521.746	2	67.74	1	0	2	.089	1	.744	3
586			min	-321.111	2	-587.882	3	3.272	15	0	3	.004	15	-.964	2
587		9	max	577.511	3	52.771	2	104.084	1	0	3	-.003	15	.867	3
588			min	-259.457	2	.378	15	5.022	15	0	9	-.054	1	-1.103	2
589		10	max	578.048	3	51.53	2	104.084	1	0	3	0	1	.847	3
590			min	-258.74	2	.004	15	5.022	15	0	9	0	10	-1.131	2
591		11	max	578.585	3	50.29	2	104.084	1	0	3	.056	1	.827	3
592			min	-258.024	2	-1.529	4	5.022	15	0	9	.003	15	-1.158	2
593		12	max	592.093	3	392.306	3	66.431	1	0	3	-.004	15	.723	3
594			min	-196.281	2	-632.113	2	3.196	15	0	2	-.088	1	-1.028	2
595		13	max	592.63	3	391.375	3	66.431	1	0	3	-.003	15	.516	3
596			min	-195.565	2	-633.354	2	3.196	15	0	2	-.053	1	-.694	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	593.167	3	390.445	3	66.431	1	0	3	0	15	.31	3
598		min	-194.848	2	-634.594	2	3.196	15	0	2	-.018	1	-.359	2
599	15	max	593.705	3	389.515	3	66.431	1	0	3	.018	1	.104	3
600		min	-194.132	2	-635.835	2	3.196	15	0	2	0	15	-.04	1
601	16	max	594.242	3	388.584	3	66.431	1	0	3	.053	1	.312	2
602		min	-193.416	2	-637.075	2	3.196	15	0	2	.003	15	-.101	3
603	17	max	594.779	3	387.654	3	66.431	1	0	3	.088	1	.648	2
604		min	-192.7	2	-638.316	2	3.196	15	0	2	.004	15	-.306	3
605	18	max	-7.051	15	637.827	2	73.508	1	0	2	.125	1	.327	2
606		min	-144.518	1	-309.941	3	3.543	15	0	3	.006	15	-.151	3
607	19	max	-6.835	15	636.586	2	73.508	1	0	2	.163	1	.013	3
608		min	-143.802	1	-310.872	3	3.543	15	0	3	.008	15	-.009	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.118	2	.01	3	9.859e-3	2	NC	1	NC	1
2			min	0	15	-.027	3	-.006	2	-2.545e-3	3	NC	1	NC	1
3		2	max	0	1	.164	3	.017	1	1.098e-2	2	NC	4	NC	1
4			min	0	15	0	15	-.002	10	-2.491e-3	3	1009.49	3	NC	1
5		3	max	0	1	.318	3	.041	1	1.21e-2	2	NC	5	NC	2
6			min	0	15	-.039	1	0	10	-2.437e-3	3	557.057	3	4588.351	1
7		4	max	0	1	.413	3	.062	1	1.322e-2	2	NC	5	NC	3
8			min	0	15	-.069	1	.002	10	-2.384e-3	3	436.96	3	3090.064	1
9		5	max	0	1	.437	3	.071	1	1.434e-2	2	NC	5	NC	3
10			min	0	15	-.067	1	.001	10	-2.33e-3	3	414.57	3	2671.574	1
11		6	max	0	1	.391	3	.068	1	1.546e-2	2	NC	5	NC	3
12			min	0	15	-.032	1	0	10	-2.277e-3	3	460.026	3	2817.486	1
13		7	max	0	1	.29	3	.051	1	1.658e-2	2	NC	4	NC	2
14			min	0	15	0	15	-.004	10	-2.223e-3	3	606.859	3	3704.262	1
15		8	max	0	1	.161	3	.03	3	1.77e-2	2	NC	1	NC	2
16			min	0	15	.002	15	-.008	10	-2.169e-3	3	1025.568	3	6968.694	1
17		9	max	0	1	.204	2	.03	3	1.882e-2	2	NC	4	NC	1
18			min	0	15	.004	15	-.016	2	-2.116e-3	3	2236.848	2	9489.049	3
19		10	max	0	1	.236	2	.03	3	1.994e-2	2	NC	3	NC	1
20		min	0	1	-.01	3	-.021	2	-2.062e-3	3	1629.2	2	9560.432	3	
21	11	max	0	15	.204	2	.03	3	1.882e-2	2	NC	4	NC	1	
22		min	0	1	.004	15	-.016	2	-2.116e-3	3	2236.848	2	9489.049	3	
23	12	max	0	15	.161	3	.03	3	1.77e-2	2	NC	1	NC	2	
24		min	0	1	.002	15	-.008	10	-2.169e-3	3	1025.568	3	6968.694	1	
25	13	max	0	15	.29	3	.051	1	1.658e-2	2	NC	4	NC	2	
26		min	0	1	0	15	-.004	10	-2.223e-3	3	606.859	3	3704.262	1	
27	14	max	0	15	.391	3	.068	1	1.546e-2	2	NC	5	NC	3	
28		min	0	1	-.032	1	0	10	-2.277e-3	3	460.026	3	2817.486	1	
29	15	max	0	15	.437	3	.071	1	1.434e-2	2	NC	5	NC	3	
30		min	0	1	-.067	1	.001	10	-2.33e-3	3	414.57	3	2671.574	1	
31	16	max	0	15	.413	3	.062	1	1.322e-2	2	NC	5	NC	3	
32		min	0	1	-.069	1	.002	10	-2.384e-3	3	436.96	3	3090.064	1	
33	17	max	0	15	.318	3	.041	1	1.21e-2	2	NC	5	NC	2	
34		min	0	1	-.039	1	0	10	-2.437e-3	3	557.057	3	4588.351	1	
35	18	max	0	15	.164	3	.017	1	1.098e-2	2	NC	4	NC	1	
36		min	0	1	0	15	-.002	10	-2.491e-3	3	1009.49	3	NC	1	
37	19	max	0	15	.118	2	.01	3	9.859e-3	2	NC	1	NC	1	
38		min	0	1	-.027	3	-.006	2	-2.545e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.265	3	.009	3	5.58e-3	2	NC	1	NC	1
40			min	0	15	-.373	2	-.005	2	-4.573e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.48	3	.011	1	6.546e-3	2	NC	5	NC	1
42		min	0	15	-.568	2	-.002	10	-5.447e-3	3	891.609	3	NC	1
43	3	max	0	1	.666	3	.032	1	7.512e-3	2	NC	5	NC	2
44		min	0	15	-.742	2	0	10	-6.322e-3	3	478.06	3	5922.449	1
45	4	max	0	1	.804	3	.052	1	8.479e-3	2	NC	5	NC	2
46		min	0	15	-.877	2	0	10	-7.196e-3	3	356.388	3	3703.546	1
47	5	max	0	1	.881	3	.062	1	9.445e-3	2	NC	5	NC	3
48		min	0	15	-.967	2	.001	10	-8.071e-3	3	311.514	3	3073.035	1
49	6	max	0	1	.899	3	.061	1	1.041e-2	2	NC	5	NC	2
50		min	0	15	-1.009	2	0	10	-8.945e-3	3	301.957	2	3156.09	1
51	7	max	0	1	.865	3	.047	1	1.138e-2	2	NC	5	NC	2
52		min	0	15	-1.009	2	-.003	10	-9.82e-3	3	301.946	2	4069.777	1
53	8	max	0	1	.8	3	.027	3	1.234e-2	2	NC	5	NC	2
54		min	0	15	-.98	2	-.007	10	-1.069e-2	3	316.366	2	7519.124	1
55	9	max	0	1	.731	3	.027	3	1.331e-2	2	NC	5	NC	1
56		min	0	15	-.942	2	-.014	2	-1.157e-2	3	337.522	2	NC	1
57	10	max	0	1	.698	3	.027	3	1.428e-2	2	NC	5	NC	1
58		min	0	1	-.922	2	-.019	2	-1.244e-2	3	349.842	2	NC	1
59	11	max	0	15	.731	3	.027	3	1.331e-2	2	NC	5	NC	1
60		min	0	1	-.942	2	-.014	2	-1.157e-2	3	337.522	2	NC	1
61	12	max	0	15	.8	3	.027	3	1.234e-2	2	NC	5	NC	2
62		min	0	1	-.98	2	-.007	10	-1.069e-2	3	316.366	2	7519.124	1
63	13	max	0	15	.865	3	.047	1	1.138e-2	2	NC	5	NC	2
64		min	0	1	-1.009	2	-.003	10	-9.82e-3	3	301.946	2	4069.777	1
65	14	max	0	15	.899	3	.061	1	1.041e-2	2	NC	5	NC	2
66		min	0	1	-1.009	2	0	10	-8.945e-3	3	301.957	2	3156.09	1
67	15	max	0	15	.881	3	.062	1	9.445e-3	2	NC	5	NC	3
68		min	0	1	-.967	2	.001	10	-8.071e-3	3	311.514	3	3073.035	1
69	16	max	0	15	.804	3	.052	1	8.479e-3	2	NC	5	NC	2
70		min	0	1	-.877	2	0	10	-7.196e-3	3	356.388	3	3703.546	1
71	17	max	0	15	.666	3	.032	1	7.512e-3	2	NC	5	NC	2
72		min	0	1	-.742	2	0	10	-6.322e-3	3	478.06	3	5922.449	1
73	18	max	0	15	.48	3	.011	1	6.546e-3	2	NC	5	NC	1
74		min	0	1	-.568	2	-.002	10	-5.447e-3	3	891.609	3	NC	1
75	19	max	0	15	.265	3	.009	3	5.58e-3	2	NC	1	NC	1
76		min	0	1	-.373	2	-.005	2	-4.573e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.27	.008	3	3.98e-3	3	NC	1	NC	1
78		min	0	1	-.372	2	-.005	2	-5.839e-3	2	NC	1	NC	1
79	2	max	0	15	.42	3	.011	1	4.743e-3	3	NC	5	NC	1
80		min	0	1	-.619	2	-.002	10	-6.857e-3	2	778.084	2	NC	1
81	3	max	0	15	.554	3	.032	1	5.506e-3	3	NC	5	NC	2
82		min	0	1	-.834	2	0	10	-7.874e-3	2	415.913	2	5898.504	1
83	4	max	0	15	.66	3	.052	1	6.269e-3	3	NC	5	NC	2
84		min	0	1	-.994	2	.001	10	-8.892e-3	2	308.452	2	3689.929	1
85	5	max	0	15	.731	3	.063	1	7.032e-3	3	NC	5	NC	3
86		min	0	1	-1.09	2	.002	10	-9.909e-3	2	267.524	2	3061.019	1
87	6	max	0	15	.766	3	.061	1	7.795e-3	3	NC	5	NC	3
88		min	0	1	-1.118	2	0	10	-1.093e-2	2	257.242	2	3141.001	1
89	7	max	0	15	.769	3	.047	1	8.558e-3	3	NC	5	NC	2
90		min	0	1	-1.09	2	-.003	10	-1.194e-2	2	267.341	2	4041.424	1
91	8	max	0	15	.751	3	.026	1	9.322e-3	3	NC	5	NC	2
92		min	0	1	-1.025	2	-.007	10	-1.296e-2	2	293.854	2	7414.536	1
93	9	max	0	15	.724	3	.025	3	1.008e-2	3	NC	5	NC	1
94		min	0	1	-.955	2	-.013	2	-1.398e-2	2	329.408	2	NC	1
95	10	max	0	1	.71	3	.025	3	1.085e-2	3	NC	5	NC	1
96		min	0	1	-.92	2	-.018	2	-1.5e-2	2	350.296	2	NC	1
97	11	max	0	1	.724	3	.025	3	1.008e-2	3	NC	5	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-.955	2	-.013	2	-1.398e-2	2	329.408	2	NC	1
99		max	0	1	.751	3	.026	1	9.322e-3	3	NC	5	NC	2
100		min	0	15	-1.025	2	-.007	10	-1.296e-2	2	293.854	2	7414.536	1
101		max	0	1	.769	3	.047	1	8.558e-3	3	NC	5	NC	2
102		min	0	15	-1.09	2	-.003	10	-1.194e-2	2	267.341	2	4041.424	1
103		max	0	1	.766	3	.061	1	7.795e-3	3	NC	5	NC	3
104		min	0	15	-1.118	2	0	10	-1.093e-2	2	257.242	2	3141.001	1
105		max	0	1	.731	3	.063	1	7.032e-3	3	NC	5	NC	3
106		min	0	15	-1.09	2	.002	10	-9.909e-3	2	267.524	2	3061.019	1
107		max	0	1	.66	3	.052	1	6.269e-3	3	NC	5	NC	2
108		min	0	15	-.994	2	.001	10	-8.892e-3	2	308.452	2	3689.929	1
109		max	0	1	.554	3	.032	1	5.506e-3	3	NC	5	NC	2
110		min	0	15	-.834	2	0	10	-7.874e-3	2	415.913	2	5898.504	1
111		max	0	1	.42	3	.011	1	4.743e-3	3	NC	5	NC	1
112		min	0	15	-.619	2	-.002	10	-6.857e-3	2	778.084	2	NC	1
113		max	0	1	.27	3	.008	3	3.98e-3	3	NC	1	NC	1
114		min	0	15	-.372	2	-.005	2	-5.839e-3	2	NC	1	NC	1
115	M16	max	0	15	.104	2	.007	3	7.26e-3	3	NC	1	NC	1
116		min	0	1	-.09	3	-.005	2	-8.165e-3	2	NC	1	NC	1
117		max	0	15	.004	14	.017	1	8.251e-3	3	NC	4	NC	1
118		min	0	1	-.036	2	-.001	10	-8.898e-3	2	1362.932	2	NC	1
119		max	0	15	.013	3	.042	1	9.242e-3	3	NC	5	NC	2
120		min	0	1	-.148	2	.001	10	-9.63e-3	2	760.448	2	4586.079	1
121		max	0	15	.034	3	.062	1	1.023e-2	3	NC	5	NC	3
122		min	0	1	-.211	2	.003	10	-1.036e-2	2	608.96	2	3078.888	1
123		max	0	15	.026	3	.072	1	1.122e-2	3	NC	5	NC	3
124		min	0	1	-.216	2	.003	10	-1.109e-2	2	599.308	2	2652.846	1
125		max	0	15	0	15	.069	1	1.221e-2	3	NC	5	NC	3
126		min	0	1	-.165	2	.002	10	-1.183e-2	2	712.709	2	2783.719	1
127		max	0	15	.005	9	.053	1	1.321e-2	3	NC	4	NC	2
128		min	0	1	-.07	2	-.001	10	-1.256e-2	2	1099.314	2	3624.277	1
129		max	0	15	.058	1	.029	1	1.42e-2	3	NC	4	NC	2
130		min	0	1	-.124	3	-.005	10	-1.329e-2	2	3251.769	2	6627.22	1
131		max	0	15	.148	2	.022	3	1.519e-2	3	NC	4	NC	1
132		min	0	1	-.179	3	-.012	2	-1.402e-2	2	2152.778	3	NC	1
133		max	0	1	.195	2	.021	3	1.618e-2	3	NC	4	NC	1
134		min	0	1	-.203	3	-.017	2	-1.476e-2	2	1693.518	3	NC	1
135		max	0	1	.148	2	.022	3	1.519e-2	3	NC	4	NC	1
136		min	0	15	-.179	3	-.012	2	-1.402e-2	2	2152.778	3	NC	1
137		max	0	1	.058	1	.029	1	1.42e-2	3	NC	4	NC	2
138		min	0	15	-.124	3	-.005	10	-1.329e-2	2	3251.769	2	6627.22	1
139		max	0	1	.005	9	.053	1	1.321e-2	3	NC	4	NC	2
140		min	0	15	-.07	2	-.001	10	-1.256e-2	2	1099.314	2	3624.277	1
141		max	0	1	0	15	.069	1	1.221e-2	3	NC	5	NC	3
142		min	0	15	-.165	2	.002	10	-1.183e-2	2	712.709	2	2783.719	1
143		max	0	1	.026	3	.072	1	1.122e-2	3	NC	5	NC	3
144		min	0	15	-.216	2	.003	10	-1.109e-2	2	599.308	2	2652.846	1
145		max	0	1	.034	3	.062	1	1.023e-2	3	NC	5	NC	3
146		min	0	15	-.211	2	.003	10	-1.036e-2	2	608.96	2	3078.888	1
147		max	0	1	.013	3	.042	1	9.242e-3	3	NC	5	NC	2
148		min	0	15	-.148	2	.001	10	-9.63e-3	2	760.448	2	4586.079	1
149		max	0	1	.004	14	.017	1	8.251e-3	3	NC	4	NC	1
150		min	0	15	-.036	2	-.001	10	-8.898e-3	2	1362.932	2	NC	1
151		max	0	1	.104	2	.007	3	7.26e-3	3	NC	1	NC	1
152		min	0	15	-.09	3	-.005	2	-8.165e-3	2	NC	1	NC	1
153	M2	max	.007	2	.009	2	.005	1	-6.899e-6	15	NC	1	NC	1
154		min	-.01	3	-.014	3	0	15	-1.425e-4	1	7655.821	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.007	2	.008	2	.005	1	-6.512e-6	15	NC	1	NC	1
156		min	-.009	3	-.014	3	0	15	-1.345e-4	1	8794.107	2	NC	1
157	3	max	.006	2	.007	2	.004	1	-6.125e-6	15	NC	1	NC	1
158		min	-.009	3	-.013	3	0	15	-1.265e-4	1	NC	1	NC	1
159	4	max	.006	2	.006	2	.004	1	-5.737e-6	15	NC	1	NC	1
160		min	-.008	3	-.013	3	0	15	-1.185e-4	1	NC	1	NC	1
161	5	max	.005	2	.005	2	.004	1	-5.35e-6	15	NC	1	NC	1
162		min	-.008	3	-.012	3	0	15	-1.105e-4	1	NC	1	NC	1
163	6	max	.005	2	.004	2	.003	1	-4.963e-6	15	NC	1	NC	1
164		min	-.007	3	-.012	3	0	15	-1.025e-4	1	NC	1	NC	1
165	7	max	.005	2	.003	2	.003	1	-4.575e-6	15	NC	1	NC	1
166		min	-.007	3	-.011	3	0	15	-9.445e-5	1	NC	1	NC	1
167	8	max	.004	2	.002	2	.002	1	-4.188e-6	15	NC	1	NC	1
168		min	-.006	3	-.01	3	0	15	-8.644e-5	1	NC	1	NC	1
169	9	max	.004	2	0	2	.002	1	-3.8e-6	15	NC	1	NC	1
170		min	-.005	3	-.01	3	0	15	-7.842e-5	1	NC	1	NC	1
171	10	max	.003	2	0	2	.002	1	-3.413e-6	15	NC	1	NC	1
172		min	-.005	3	-.009	3	0	15	-7.041e-5	1	NC	1	NC	1
173	11	max	.003	2	0	2	.001	1	-3.026e-6	15	NC	1	NC	1
174		min	-.004	3	-.008	3	0	15	-6.24e-5	1	NC	1	NC	1
175	12	max	.003	2	0	2	.001	1	-2.638e-6	15	NC	1	NC	1
176		min	-.004	3	-.007	3	0	15	-5.438e-5	1	NC	1	NC	1
177	13	max	.002	2	-.001	15	0	1	-2.251e-6	15	NC	1	NC	1
178		min	-.003	3	-.006	3	0	15	-4.637e-5	1	NC	1	NC	1
179	14	max	.002	2	-.001	15	0	1	-1.864e-6	15	NC	1	NC	1
180		min	-.003	3	-.005	3	0	15	-3.836e-5	1	NC	1	NC	1
181	15	max	.002	2	0	15	0	1	-1.476e-6	15	NC	1	NC	1
182		min	-.002	3	-.005	3	0	15	-3.034e-5	1	NC	1	NC	1
183	16	max	.001	2	0	15	0	1	-1.089e-6	15	NC	1	NC	1
184		min	-.002	3	-.003	3	0	15	-2.233e-5	1	NC	1	NC	1
185	17	max	0	2	0	15	0	1	-7.016e-7	15	NC	1	NC	1
186		min	-.001	3	-.002	3	0	15	-1.431e-5	1	NC	1	NC	1
187	18	max	0	2	0	15	0	1	-3.142e-7	15	NC	1	NC	1
188		min	0	3	-.001	4	0	15	-6.3e-6	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.714e-6	1	NC	1	NC	1
190		min	0	1	0	1	0	1	-3.57e-7	3	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	0	3	NC	1	NC	1
192		min	0	1	0	1	0	1	-1.191e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	1.316e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	3	6.336e-7	15	NC	1	NC	1
195	3	max	0	3	0	15	0	1	2.751e-5	1	NC	1	NC	1
196		min	0	2	-.004	4	0	3	1.322e-6	15	NC	1	NC	1
197	4	max	.001	3	-.001	15	0	1	4.186e-5	1	NC	1	NC	1
198		min	-.001	2	-.006	4	0	12	2.011e-6	15	NC	1	NC	1
199	5	max	.002	3	-.002	15	0	1	5.621e-5	1	NC	1	NC	1
200		min	-.002	2	-.008	4	0	12	2.699e-6	15	NC	1	NC	1
201	6	max	.002	3	-.002	15	0	1	7.056e-5	1	NC	1	NC	1
202		min	-.002	2	-.01	4	0	12	3.388e-6	15	9638.578	4	NC	1
203	7	max	.003	3	-.003	15	0	1	8.491e-5	1	NC	1	NC	1
204		min	-.002	2	-.011	4	0	15	4.076e-6	15	8301.533	4	NC	1
205	8	max	.003	3	-.003	15	0	1	9.926e-5	1	NC	1	NC	1
206		min	-.003	2	-.012	4	0	15	4.765e-6	15	7477.48	4	NC	1
207	9	max	.004	3	-.003	15	0	1	1.136e-4	1	NC	2	NC	1
208		min	-.003	2	-.013	4	0	15	5.453e-6	15	6993.215	4	NC	1
209	10	max	.004	3	-.003	15	0	1	1.28e-4	1	NC	5	NC	1
210		min	-.003	2	-.014	4	0	15	6.142e-6	15	6764.974	4	NC	1
211	11	max	.004	3	-.003	15	.001	1	1.423e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.014	4	0	15	6.83e-6	15	6759.314	4	NC	1
213		max	.005	3	-.003	15	.001	1	1.567e-4	1	NC	2	NC	1
214		min	-.004	2	-.013	4	0	15	7.519e-6	15	6980.301	4	NC	1
215		max	.005	3	-.003	15	.002	1	1.71e-4	1	NC	1	NC	1
216		min	-.005	2	-.013	4	0	15	8.207e-6	15	7472.726	4	NC	1
217		max	.006	3	-.003	15	.002	1	1.854e-4	1	NC	1	NC	1
218		min	-.005	2	-.011	4	0	15	8.896e-6	15	8345.045	4	NC	1
219		max	.006	3	-.002	15	.003	1	1.997e-4	1	NC	1	NC	1
220		min	-.005	2	-.01	4	0	15	9.584e-6	15	9836.591	4	NC	1
221		max	.007	3	-.002	15	.003	1	2.141e-4	1	NC	1	NC	1
222		min	-.006	2	-.008	4	0	15	1.027e-5	15	NC	1	NC	1
223		max	.007	3	-.001	15	.004	1	2.284e-4	1	NC	1	NC	1
224		min	-.006	2	-.006	4	0	15	1.096e-5	15	NC	1	NC	1
225		max	.007	3	0	15	.004	1	2.428e-4	1	NC	1	NC	1
226		min	-.006	2	-.004	3	0	15	1.165e-5	15	NC	1	NC	1
227		max	.008	3	0	10	.005	1	2.571e-4	1	NC	1	NC	1
228		min	-.007	2	-.002	3	0	15	1.234e-5	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	6.894e-5	1	NC	1	NC	2
230		min	0	3	-.008	3	-.005	1	3.326e-6	15	NC	1	4885.848	1
231		max	.002	1	.006	2	0	15	6.894e-5	1	NC	1	NC	2
232		min	0	3	-.008	3	-.005	1	3.326e-6	15	NC	1	5303.65	1
233		max	.002	1	.006	2	0	15	6.894e-5	1	NC	1	NC	2
234		min	0	3	-.007	3	-.004	1	3.326e-6	15	NC	1	5801.479	1
235		max	.002	1	.005	2	0	15	6.894e-5	1	NC	1	NC	2
236		min	0	3	-.007	3	-.004	1	3.326e-6	15	NC	1	6400.021	1
237		max	.002	1	.005	2	0	15	6.894e-5	1	NC	1	NC	2
238		min	0	3	-.006	3	-.003	1	3.326e-6	15	NC	1	7127.395	1
239		max	.002	1	.005	2	0	15	6.894e-5	1	NC	1	NC	2
240		min	0	3	-.006	3	-.003	1	3.326e-6	15	NC	1	8022.598	1
241		max	.001	1	.004	2	0	15	6.894e-5	1	NC	1	NC	2
242		min	0	3	-.005	3	-.003	1	3.326e-6	15	NC	1	9140.986	1
243		max	.001	1	.004	2	0	15	6.894e-5	1	NC	1	NC	1
244		min	0	3	-.005	3	-.002	1	3.326e-6	15	NC	1	NC	1
245		max	.001	1	.004	2	0	15	6.894e-5	1	NC	1	NC	1
246		min	0	3	-.005	3	-.002	1	3.326e-6	15	NC	1	NC	1
247		max	.001	1	.003	2	0	15	6.894e-5	1	NC	1	NC	1
248		min	0	3	-.004	3	-.002	1	3.326e-6	15	NC	1	NC	1
249		max	0	1	.003	2	0	15	6.894e-5	1	NC	1	NC	1
250		min	0	3	-.004	3	-.001	1	3.326e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	6.894e-5	1	NC	1	NC	1
252		min	0	3	-.003	3	-.001	1	3.326e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	6.894e-5	1	NC	1	NC	1
254		min	0	3	-.003	3	0	1	3.326e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	6.894e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	3.326e-6	15	NC	1	NC	1
257		max	0	1	.001	2	0	15	6.894e-5	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	3.326e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	6.894e-5	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	3.326e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.894e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	3.326e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.894e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	3.326e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.894e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	3.326e-6	15	NC	1	NC	1
267	M6	max	.022	2	.032	2	0	1	0	1	NC	4	NC	1
268		min	-.031	3	-.045	3	0	1	0	1	1559.691	3	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.021	2	.029	2	0	1	0	1	NC	4	NC	1
270		min	-.03	3	-.042	3	0	1	0	1	1653.765	3	NC	1
271	3	max	.019	2	.026	2	0	1	0	1	NC	4	NC	1
272		min	-.028	3	-.04	3	0	1	0	1	1759.943	3	NC	1
273	4	max	.018	2	.024	2	0	1	0	1	NC	4	NC	1
274		min	-.026	3	-.037	3	0	1	0	1	1880.73	3	NC	1
275	5	max	.017	2	.021	2	0	1	0	1	NC	4	NC	1
276		min	-.024	3	-.035	3	0	1	0	1	2019.351	3	NC	1
277	6	max	.016	2	.019	2	0	1	0	1	NC	4	NC	1
278		min	-.023	3	-.032	3	0	1	0	1	2180.025	3	NC	1
279	7	max	.015	2	.016	2	0	1	0	1	NC	1	NC	1
280		min	-.021	3	-.03	3	0	1	0	1	2368.381	3	NC	1
281	8	max	.013	2	.014	2	0	1	0	1	NC	1	NC	1
282		min	-.019	3	-.027	3	0	1	0	1	2592.101	3	NC	1
283	9	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
284		min	-.017	3	-.024	3	0	1	0	1	2861.943	3	NC	1
285	10	max	.011	2	.01	2	0	1	0	1	NC	1	NC	1
286		min	-.016	3	-.022	3	0	1	0	1	3193.458	3	NC	1
287	11	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
288		min	-.014	3	-.019	3	0	1	0	1	3609.983	3	NC	1
289	12	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.012	3	-.017	3	0	1	0	1	4148.205	3	NC	1
291	13	max	.007	2	.005	2	0	1	0	1	NC	1	NC	1
292		min	-.01	3	-.014	3	0	1	0	1	4869.293	3	NC	1
293	14	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.009	3	-.012	3	0	1	0	1	5883.383	3	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.007	3	-.009	3	0	1	0	1	7410.792	3	NC	1
297	16	max	.004	2	.001	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.007	3	0	1	0	1	9965.667	3	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	0	2	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.007	2	-.012	3	0	1	0	1	8623.702	3	NC	1
317	7	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.014	3	0	1	0	1	7709.723	3	NC	1
319	8	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7170.054	3	NC	1
321	9	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.01	2	-.016	3	0	1	0	1	6893.064	3	NC	1
323	10	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.012	2	-.016	3	0	1	0	1	6828.846	3	NC	1
325	11	max	.013	3	-.003	15	0	1	0	1	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.016	3	0	1	0	1	6839.847	4	NC	1
327		12	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.014	2	-.016	3	0	1	0	1	7059.745	4	NC	1
329		13	max	.016	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.016	2	-.016	3	0	1	0	1	7554.419	4	NC	1
331		14	max	.018	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.017	2	-.015	3	0	1	0	1	8433.155	4	NC	1
333		15	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.018	2	-.013	3	0	1	0	1	9937.445	4	NC	1
335		16	max	.02	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.02	2	-.012	3	0	1	0	1	NC	1	NC	1
337		17	max	.022	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.021	2	-.01	3	0	1	0	1	NC	1	NC	1
339		18	max	.023	3	0	10	0	1	0	1	NC	1	NC	1
340			min	-.022	2	-.008	3	0	1	0	1	NC	1	NC	1
341		19	max	.024	3	0	10	0	1	0	1	NC	1	NC	1
342			min	-.023	2	-.006	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	2	.023	2	0	1	0	1	NC	1	NC	1
344			min	0	3	-.025	3	0	1	0	1	NC	1	NC	1
345		2	max	.005	2	.021	2	0	1	0	1	NC	1	NC	1
346			min	0	3	-.024	3	0	1	0	1	NC	1	NC	1
347		3	max	.005	2	.02	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.022	3	0	1	0	1	NC	1	NC	1
349		4	max	.005	2	.019	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.021	3	0	1	0	1	NC	1	NC	1
351		5	max	.005	2	.018	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.02	3	0	1	0	1	NC	1	NC	1
353		6	max	.004	2	.016	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
355		7	max	.004	2	.015	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.017	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	2	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
359		9	max	.003	2	.013	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	2	.011	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	2	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	2	.008	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.001	2	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	0	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.009	2	0	15	1.425e-4	1	NC	1	NC	1
382			min	-.01	3	-.014	3	-.005	1	6.899e-6	15	7655.821	2	NC	1





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Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.007	2	.008	2	0	15	1.345e-4	1	NC	1	NC	1
384			min	-.009	3	-.014	3	-.005	1	6.512e-6	15	8794.107	2	NC	1
385		3	max	.006	2	.007	2	0	15	1.265e-4	1	NC	1	NC	1
386			min	-.009	3	-.013	3	-.004	1	6.125e-6	15	NC	1	NC	1
387		4	max	.006	2	.006	2	0	15	1.185e-4	1	NC	1	NC	1
388			min	-.008	3	-.013	3	-.004	1	5.737e-6	15	NC	1	NC	1
389		5	max	.005	2	.005	2	0	15	1.105e-4	1	NC	1	NC	1
390			min	-.008	3	-.012	3	-.004	1	5.35e-6	15	NC	1	NC	1
391		6	max	.005	2	.004	2	0	15	1.025e-4	1	NC	1	NC	1
392			min	-.007	3	-.012	3	-.003	1	4.963e-6	15	NC	1	NC	1
393		7	max	.005	2	.003	2	0	15	9.445e-5	1	NC	1	NC	1
394			min	-.007	3	-.011	3	-.003	1	4.575e-6	15	NC	1	NC	1
395		8	max	.004	2	.002	2	0	15	8.644e-5	1	NC	1	NC	1
396			min	-.006	3	-.01	3	-.002	1	4.188e-6	15	NC	1	NC	1
397		9	max	.004	2	0	2	0	15	7.842e-5	1	NC	1	NC	1
398			min	-.005	3	-.01	3	-.002	1	3.8e-6	15	NC	1	NC	1
399		10	max	.003	2	0	2	0	15	7.041e-5	1	NC	1	NC	1
400			min	-.005	3	-.009	3	-.002	1	3.413e-6	15	NC	1	NC	1
401		11	max	.003	2	0	2	0	15	6.24e-5	1	NC	1	NC	1
402			min	-.004	3	-.008	3	-.001	1	3.026e-6	15	NC	1	NC	1
403		12	max	.003	2	0	2	0	15	5.438e-5	1	NC	1	NC	1
404			min	-.004	3	-.007	3	-.001	1	2.638e-6	15	NC	1	NC	1
405		13	max	.002	2	-.001	15	0	15	4.637e-5	1	NC	1	NC	1
406			min	-.003	3	-.006	3	0	1	2.251e-6	15	NC	1	NC	1
407		14	max	.002	2	-.001	15	0	15	3.836e-5	1	NC	1	NC	1
408			min	-.003	3	-.005	3	0	1	1.864e-6	15	NC	1	NC	1
409		15	max	.002	2	0	15	0	15	3.034e-5	1	NC	1	NC	1
410			min	-.002	3	-.005	3	0	1	1.476e-6	15	NC	1	NC	1
411		16	max	.001	2	0	15	0	15	2.233e-5	1	NC	1	NC	1
412			min	-.002	3	-.003	3	0	1	1.089e-6	15	NC	1	NC	1
413		17	max	0	2	0	15	0	15	1.431e-5	1	NC	1	NC	1
414			min	-.001	3	-.002	3	0	1	7.016e-7	15	NC	1	NC	1
415		18	max	0	2	0	15	0	15	6.3e-6	1	NC	1	NC	1
416			min	0	3	-.001	4	0	1	3.142e-7	15	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.57e-7	3	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.714e-6	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.191e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	0	3	NC	1	NC	1
421		2	max	0	3	0	15	0	3	-6.336e-7	15	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.316e-5	1	NC	1	NC	1
423		3	max	0	3	0	15	0	3	-1.322e-6	15	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-2.751e-5	1	NC	1	NC	1
425		4	max	.001	3	-.001	15	0	12	-2.011e-6	15	NC	1	NC	1
426			min	-.001	2	-.006	4	0	1	-4.186e-5	1	NC	1	NC	1
427		5	max	.002	3	-.002	15	0	12	-2.699e-6	15	NC	1	NC	1
428			min	-.002	2	-.008	4	0	1	-5.621e-5	1	NC	1	NC	1
429		6	max	.002	3	-.002	15	0	12	-3.388e-6	15	NC	1	NC	1
430			min	-.002	2	-.01	4	0	1	-7.056e-5	1	9638.578	4	NC	1
431		7	max	.003	3	-.003	15	0	15	-4.076e-6	15	NC	1	NC	1
432			min	-.002	2	-.011	4	0	1	-8.491e-5	1	8301.533	4	NC	1
433		8	max	.003	3	-.003	15	0	15	-4.765e-6	15	NC	1	NC	1
434			min	-.003	2	-.012	4	0	1	-9.926e-5	1	7477.48	4	NC	1
435		9	max	.004	3	-.003	15	0	15	-5.453e-6	15	NC	2	NC	1
436			min	-.003	2	-.013	4	0	1	-1.136e-4	1	6993.215	4	NC	1
437		10	max	.004	3	-.003	15	0	15	-6.142e-6	15	NC	5	NC	1
438			min	-.003	2	-.014	4	0	1	-1.28e-4	1	6764.974	4	NC	1
439		11	max	.004	3	-.003	15	0	15	-6.83e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.004	2	-.014	4	-.001	1	-1.423e-4	1	6759.314	4	NC	1
441		max	.005	3	-.003	15	0	15	-7.519e-6	15	NC	2	NC	1
442		min	-.004	2	-.013	4	-.001	1	-1.567e-4	1	6980.301	4	NC	1
443		max	.005	3	-.003	15	0	15	-8.207e-6	15	NC	1	NC	1
444		min	-.005	2	-.013	4	-.002	1	-1.71e-4	1	7472.726	4	NC	1
445		max	.006	3	-.003	15	0	15	-8.896e-6	15	NC	1	NC	1
446		min	-.005	2	-.011	4	-.002	1	-1.854e-4	1	8345.045	4	NC	1
447		max	.006	3	-.002	15	0	15	-9.584e-6	15	NC	1	NC	1
448		min	-.005	2	-.01	4	-.003	1	-1.997e-4	1	9836.591	4	NC	1
449		max	.007	3	-.002	15	0	15	-1.027e-5	15	NC	1	NC	1
450		min	-.006	2	-.008	4	-.003	1	-2.141e-4	1	NC	1	NC	1
451		max	.007	3	-.001	15	0	15	-1.096e-5	15	NC	1	NC	1
452		min	-.006	2	-.006	4	-.004	1	-2.284e-4	1	NC	1	NC	1
453		max	.007	3	0	15	0	15	-1.165e-5	15	NC	1	NC	1
454		min	-.006	2	-.004	3	-.004	1	-2.428e-4	1	NC	1	NC	1
455		max	.008	3	0	10	0	15	-1.234e-5	15	NC	1	NC	1
456		min	-.007	2	-.002	3	-.005	1	-2.571e-4	1	NC	1	NC	1
457	M12	max	.002	1	.007	2	.005	1	-3.326e-6	15	NC	1	NC	2
458		min	0	3	-.008	3	0	15	-6.894e-5	1	NC	1	4885.848	1
459		max	.002	1	.006	2	.005	1	-3.326e-6	15	NC	1	NC	2
460		min	0	3	-.008	3	0	15	-6.894e-5	1	NC	1	5303.65	1
461		max	.002	1	.006	2	.004	1	-3.326e-6	15	NC	1	NC	2
462		min	0	3	-.007	3	0	15	-6.894e-5	1	NC	1	5801.479	1
463		max	.002	1	.005	2	.004	1	-3.326e-6	15	NC	1	NC	2
464		min	0	3	-.007	3	0	15	-6.894e-5	1	NC	1	6400.021	1
465		max	.002	1	.005	2	.003	1	-3.326e-6	15	NC	1	NC	2
466		min	0	3	-.006	3	0	15	-6.894e-5	1	NC	1	7127.395	1
467		max	.002	1	.005	2	.003	1	-3.326e-6	15	NC	1	NC	2
468		min	0	3	-.006	3	0	15	-6.894e-5	1	NC	1	8022.598	1
469		max	.001	1	.004	2	.003	1	-3.326e-6	15	NC	1	NC	2
470		min	0	3	-.005	3	0	15	-6.894e-5	1	NC	1	9140.986	1
471		max	.001	1	.004	2	.002	1	-3.326e-6	15	NC	1	NC	1
472		min	0	3	-.005	3	0	15	-6.894e-5	1	NC	1	NC	1
473		max	.001	1	.004	2	.002	1	-3.326e-6	15	NC	1	NC	1
474		min	0	3	-.005	3	0	15	-6.894e-5	1	NC	1	NC	1
475		max	.001	1	.003	2	.002	1	-3.326e-6	15	NC	1	NC	1
476		min	0	3	-.004	3	0	15	-6.894e-5	1	NC	1	NC	1
477		max	0	1	.003	2	.001	1	-3.326e-6	15	NC	1	NC	1
478		min	0	3	-.004	3	0	15	-6.894e-5	1	NC	1	NC	1
479		max	0	1	.003	2	.001	1	-3.326e-6	15	NC	1	NC	1
480		min	0	3	-.003	3	0	15	-6.894e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-3.326e-6	15	NC	1	NC	1
482		min	0	3	-.003	3	0	15	-6.894e-5	1	NC	1	NC	1
483		max	0	1	.002	2	0	1	-3.326e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-6.894e-5	1	NC	1	NC	1
485		max	0	1	.001	2	0	1	-3.326e-6	15	NC	1	NC	1
486		min	0	3	-.002	3	0	15	-6.894e-5	1	NC	1	NC	1
487		max	0	1	.001	2	0	1	-3.326e-6	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-6.894e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-3.326e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-6.894e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-3.326e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.894e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-3.326e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.894e-5	1	NC	1	NC	1
495	M1	max	.01	3	.118	2	0	1	8.33e-3	2	NC	1	NC	1
496		min	-.006	2	-.027	3	0	15	-1.851e-2	3	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.01	3	.055	2	0	15	4.087e-3	2	NC	4	NC	1
498		min	-.006	2	-.01	3	-.004	1	-9.159e-3	3	1841.47	2	NC	1
499	3	max	.01	3	.015	3	0	15	2.898e-5	10	NC	5	NC	1
500		min	-.006	2	-.012	2	-.005	1	-1.093e-4	3	890.615	2	NC	1
501	4	max	.01	3	.055	3	0	15	3.485e-3	2	NC	5	NC	1
502		min	-.006	2	-.086	2	-.005	1	-3.928e-3	3	565.056	2	NC	1
503	5	max	.009	3	.105	3	0	15	6.951e-3	2	NC	5	NC	1
504		min	-.006	2	-.163	2	-.003	1	-7.747e-3	3	409.57	2	NC	1
505	6	max	.009	3	.158	3	0	15	1.042e-2	2	NC	15	NC	1
506		min	-.006	2	-.238	2	-.001	1	-1.157e-2	3	323.655	2	NC	1
507	7	max	.009	3	.209	3	0	1	1.388e-2	2	NC	15	NC	1
508		min	-.005	2	-.304	2	0	3	-1.539e-2	3	272.81	2	NC	1
509	8	max	.009	3	.252	3	0	1	1.735e-2	2	NC	15	NC	1
510		min	-.005	2	-.357	2	0	15	-1.92e-2	3	242.676	2	NC	1
511	9	max	.009	3	.279	3	0	15	1.967e-2	2	NC	15	NC	1
512		min	-.005	2	-.39	2	0	1	-1.954e-2	3	226.967	2	NC	1
513	10	max	.008	3	.289	3	0	1	2.123e-2	2	9952.129	15	NC	1
514		min	-.005	2	-.401	2	0	15	-1.757e-2	3	222.374	2	NC	1
515	11	max	.008	3	.282	3	0	1	2.279e-2	2	NC	15	NC	1
516		min	-.005	2	-.39	2	0	15	-1.559e-2	3	227.804	2	NC	1
517	12	max	.008	3	.258	3	0	15	2.199e-2	2	NC	15	NC	1
518		min	-.005	2	-.355	2	0	1	-1.334e-2	3	245.199	2	NC	1
519	13	max	.008	3	.22	3	0	15	1.763e-2	2	NC	15	NC	1
520		min	-.005	2	-.3	2	0	1	-1.068e-2	3	278.891	2	NC	1
521	14	max	.008	3	.171	3	.001	1	1.328e-2	2	NC	15	NC	1
522		min	-.005	2	-.23	2	0	15	-8.016e-3	3	336.547	2	NC	1
523	15	max	.007	3	.117	3	.003	1	8.921e-3	2	NC	5	NC	1
524		min	-.005	2	-.154	2	0	15	-5.353e-3	3	435.89	2	NC	1
525	16	max	.007	3	.06	3	.005	1	4.566e-3	2	NC	5	NC	1
526		min	-.005	2	-.077	2	0	15	-2.691e-3	3	620.053	2	NC	1
527	17	max	.007	3	.005	3	.005	1	3.58e-4	1	NC	5	NC	1
528		min	-.005	2	-.006	2	0	15	-2.881e-5	3	1014.114	2	NC	1
529	18	max	.007	3	.052	2	.004	1	6.86e-3	2	NC	4	NC	1
530		min	-.005	2	-.044	3	0	15	-2.833e-3	3	2153.957	2	NC	1
531	19	max	.007	3	.104	2	0	15	1.377e-2	2	NC	1	NC	1
532		min	-.005	2	-.09	3	0	1	-5.765e-3	3	NC	1	NC	1
533	M5	1	max	.03	.236	2	0	1	0	1	NC	1	NC	1
534		min	-.021	2	-.01	3	0	1	0	1	NC	1	NC	1
535	2	max	.03	3	.108	2	0	1	0	1	NC	5	NC	1
536		min	-.021	2	.002	15	0	1	0	1	908.242	2	NC	1
537	3	max	.03	3	.048	3	0	1	0	1	NC	5	NC	1
538		min	-.021	2	-.036	2	0	1	0	1	427.028	2	NC	1
539	4	max	.029	3	.14	3	0	1	0	1	NC	15	NC	1
540		min	-.021	2	-.207	2	0	1	0	1	261.15	2	NC	1
541	5	max	.029	3	.266	3	0	1	0	1	9395.853	15	NC	1
542		min	-.02	2	-.394	2	0	1	0	1	183.687	2	NC	1
543	6	max	.028	3	.408	3	0	1	0	1	7220.621	15	NC	1
544		min	-.02	2	-.579	2	0	1	0	1	141.913	2	NC	1
545	7	max	.027	3	.546	3	0	1	0	1	5967.094	15	NC	1
546		min	-.02	2	-.746	2	0	1	0	1	117.685	2	NC	1
547	8	max	.027	3	.662	3	0	1	0	1	5239.394	15	NC	1
548		min	-.019	2	-.881	2	0	1	0	1	103.554	2	NC	1
549	9	max	.026	3	.737	3	0	1	0	1	4866.661	15	NC	1
550		min	-.019	2	-.966	2	0	1	0	1	96.292	2	NC	1
551	10	max	.026	3	.763	3	0	1	0	1	4754.46	15	NC	1
552		min	-.019	2	-.995	2	0	1	0	1	94.175	2	NC	1
553	11	max	.025	3	.743	3	0	1	0	1	4866.949	15	NC	1





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Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.018	2	-.966	2	0	1	0	1	96.674	2	NC	1
555	12	max	.024	3	.679	3	0	1	0	1	5240.057	15	NC	1
556		min	-.018	2	-.877	2	0	1	0	1	104.817	2	NC	1
557	13	max	.024	3	.575	3	0	1	0	1	5968.399	15	NC	1
558		min	-.018	2	-.734	2	0	1	0	1	120.986	2	NC	1
559	14	max	.023	3	.444	3	0	1	0	1	7223.099	15	NC	1
560		min	-.018	2	-.557	2	0	1	0	1	149.405	2	NC	1
561	15	max	.023	3	.299	3	0	1	0	1	9400.653	15	NC	1
562		min	-.017	2	-.366	2	0	1	0	1	200.147	2	NC	1
563	16	max	.022	3	.152	3	0	1	0	1	NC	15	NC	1
564		min	-.017	2	-.18	2	0	1	0	1	298.735	2	NC	1
565	17	max	.021	3	.016	3	0	1	0	1	NC	5	NC	1
566		min	-.017	2	-.02	2	0	1	0	1	520.933	2	NC	1
567	18	max	.021	3	.099	2	0	1	0	1	NC	5	NC	1
568		min	-.017	2	-.1	3	0	1	0	1	1165.484	2	NC	1
569	19	max	.021	3	.195	2	0	1	0	1	NC	1	NC	1
570		min	-.017	2	-.203	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.118	2	0	15	1.851e-2	3	NC	1	NC	1
572		min	-.006	2	-.027	3	0	1	-8.33e-3	2	NC	1	NC	1
573	2	max	.01	3	.055	2	.004	1	9.159e-3	3	NC	4	NC	1
574		min	-.006	2	-.01	3	0	15	-4.087e-3	2	1841.47	2	NC	1
575	3	max	.01	3	.015	3	.005	1	1.093e-4	3	NC	5	NC	1
576		min	-.006	2	-.012	2	0	15	-2.898e-5	10	890.615	2	NC	1
577	4	max	.01	3	.055	3	.005	1	3.928e-3	3	NC	5	NC	1
578		min	-.006	2	-.086	2	0	15	-3.485e-3	2	565.056	2	NC	1
579	5	max	.009	3	.105	3	.003	1	7.747e-3	3	NC	5	NC	1
580		min	-.006	2	-.163	2	0	15	-6.951e-3	2	409.57	2	NC	1
581	6	max	.009	3	.158	3	.001	1	1.157e-2	3	NC	15	NC	1
582		min	-.006	2	-.238	2	0	15	-1.042e-2	2	323.655	2	NC	1
583	7	max	.009	3	.209	3	0	3	1.539e-2	3	NC	15	NC	1
584		min	-.005	2	-.304	2	0	1	-1.388e-2	2	272.81	2	NC	1
585	8	max	.009	3	.252	3	0	15	1.92e-2	3	NC	15	NC	1
586		min	-.005	2	-.357	2	0	1	-1.735e-2	2	242.676	2	NC	1
587	9	max	.009	3	.279	3	0	1	1.954e-2	3	NC	15	NC	1
588		min	-.005	2	-.39	2	0	15	-1.967e-2	2	226.967	2	NC	1
589	10	max	.008	3	.289	3	0	15	1.757e-2	3	9952.129	15	NC	1
590		min	-.005	2	-.401	2	0	1	-2.123e-2	2	222.374	2	NC	1
591	11	max	.008	3	.282	3	0	15	1.559e-2	3	NC	15	NC	1
592		min	-.005	2	-.39	2	0	1	-2.279e-2	2	227.804	2	NC	1
593	12	max	.008	3	.258	3	0	1	1.334e-2	3	NC	15	NC	1
594		min	-.005	2	-.355	2	0	15	-2.199e-2	2	245.199	2	NC	1
595	13	max	.008	3	.22	3	0	1	1.068e-2	3	NC	15	NC	1
596		min	-.005	2	-.3	2	0	15	-1.763e-2	2	278.891	2	NC	1
597	14	max	.008	3	.171	3	0	15	8.016e-3	3	NC	15	NC	1
598		min	-.005	2	-.23	2	-.001	1	-1.328e-2	2	336.547	2	NC	1
599	15	max	.007	3	.117	3	0	15	5.353e-3	3	NC	5	NC	1
600		min	-.005	2	-.154	2	-.003	1	-8.921e-3	2	435.89	2	NC	1
601	16	max	.007	3	.06	3	0	15	2.691e-3	3	NC	5	NC	1
602		min	-.005	2	-.077	2	-.005	1	-4.566e-3	2	620.053	2	NC	1
603	17	max	.007	3	.005	3	0	15	2.881e-5	3	NC	5	NC	1
604		min	-.005	2	-.006	2	-.005	1	-3.58e-4	1	1014.114	2	NC	1
605	18	max	.007	3	.052	2	0	15	2.833e-3	3	NC	4	NC	1
606		min	-.005	2	-.044	3	-.004	1	-6.86e-3	2	2153.957	2	NC	1
607	19	max	.007	3	.104	2	0	1	5.765e-3	3	NC	1	NC	1
608		min	-.005	2	-.09	3	0	15	-1.377e-2	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Project:	Standard PVMax - Worst Case, 36 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

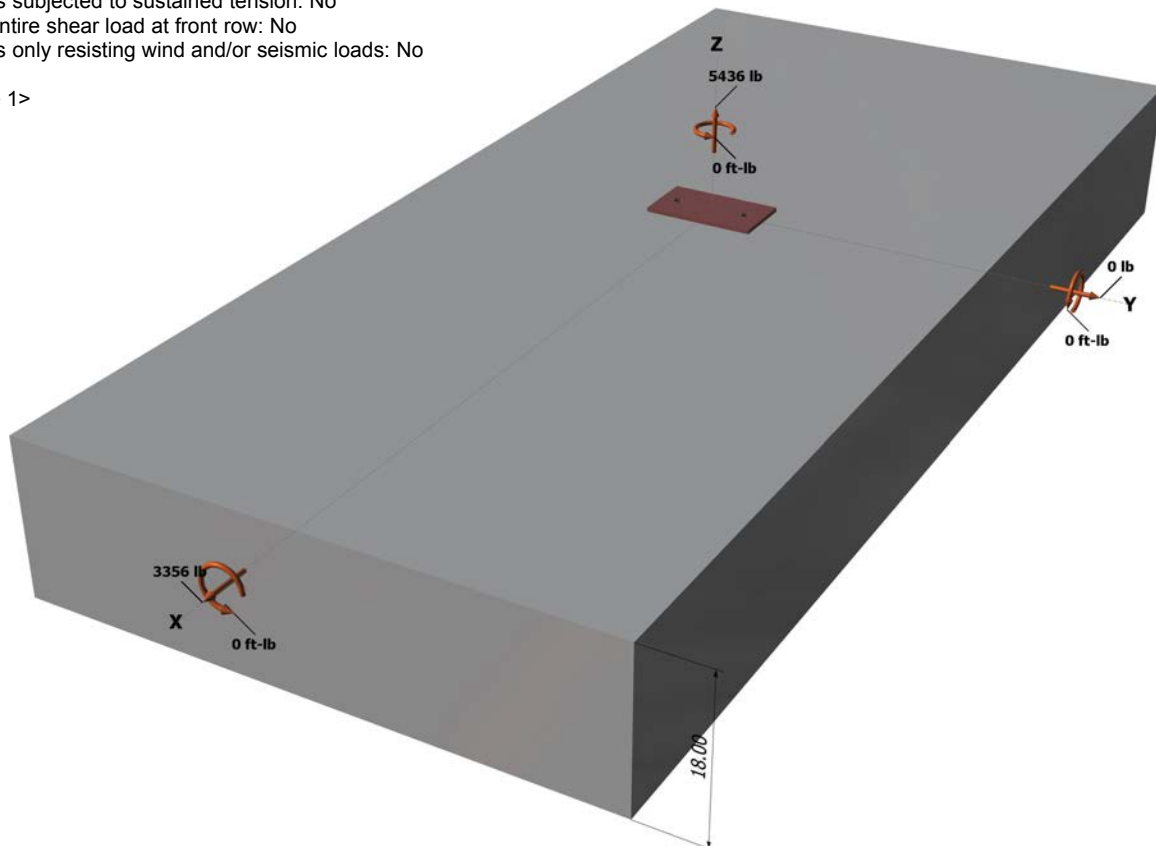
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



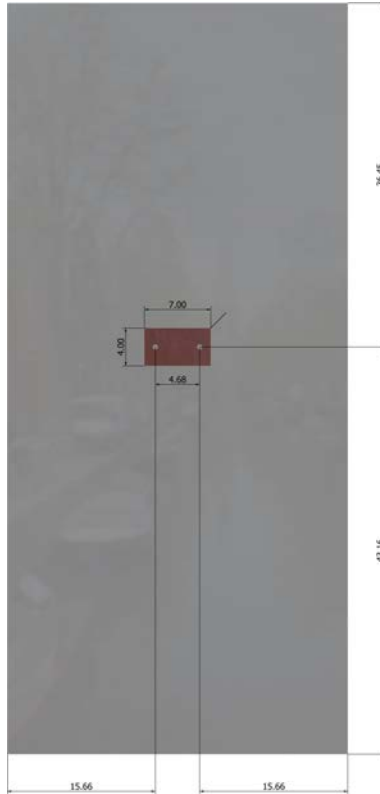
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2718.0	1678.0	0.0	1678.0
2	2718.0	1678.0	0.0	1678.0
Sum	5436.0	3356.0	0.0	3356.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5436  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
648.00	648.00	1.000	0.961	1.000	1.000	15593	0.70	10490

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	15.66	23247

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
845.64	1103.56	1.000	1.000	1.000	23247	0.70	24939

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2718	6071	0.45	Pass	
Concrete breakout	5436	10231	0.53	Pass	
<b>Adhesive</b>	<b>5436</b>	<b>8093</b>	<b>0.67</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1678</b>	<b>3156</b>	<b>0.53</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3356	10490	0.32	Pass	
Concrete breakout y-	1678	24939	0.07	Pass	
Pryout	3356	20601	0.16	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 36 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.67	0.53	120.3 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.