

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ =	1.25
$S_{DS}$ =	1.67	$C_s$ =	0.8
$S_1$ =	1.00	$\rho$ =	1.3
$S_{D1}$ =	1.00	$\Omega$ =	1.25
$T_a$ =	0.05	$C_d$ =	1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

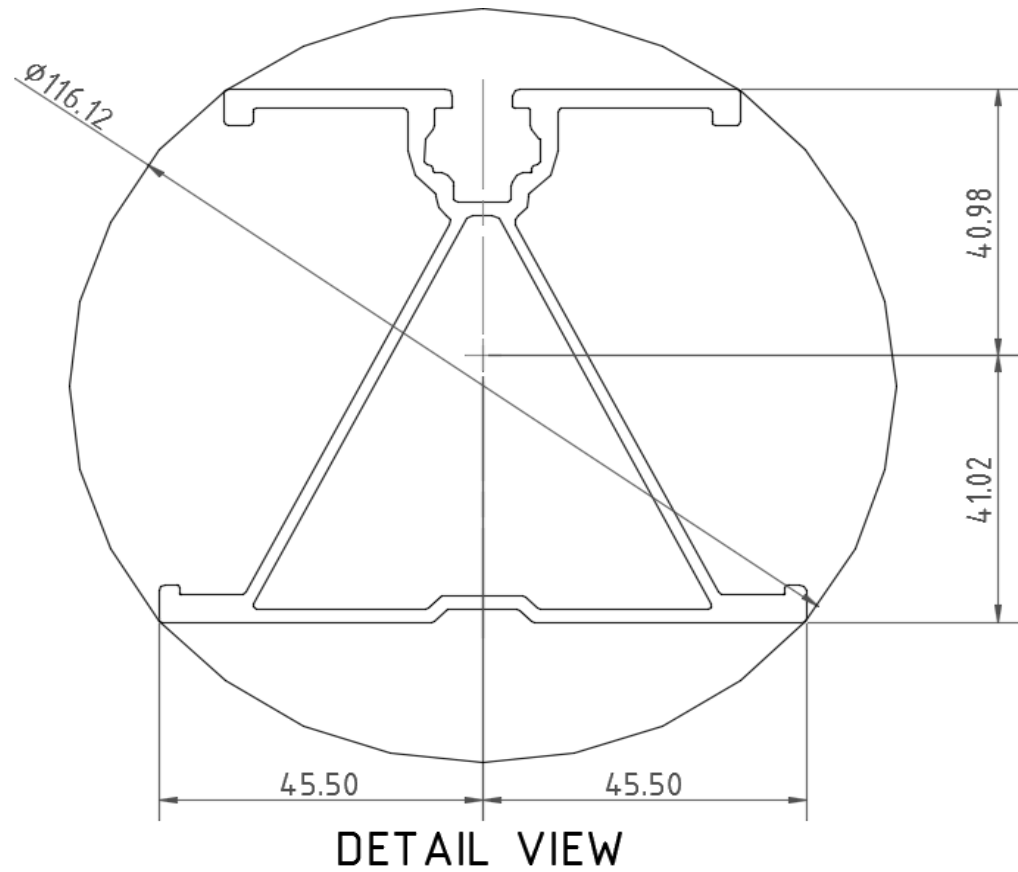
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

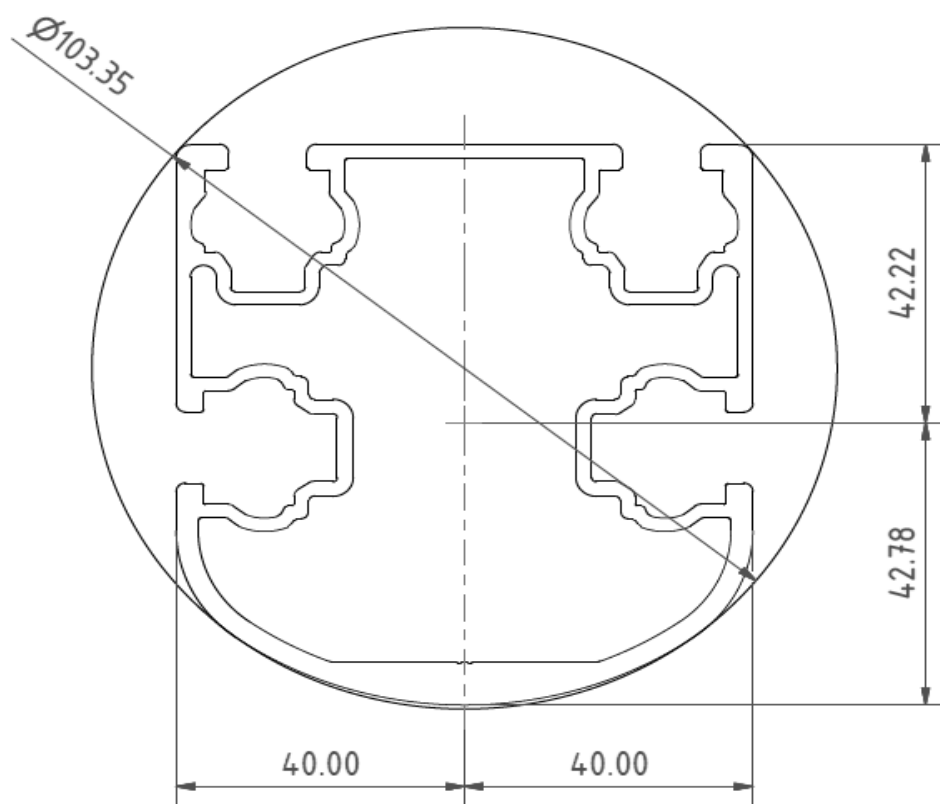
Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	120 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.119 k-ft
$M_z$ =	0.255 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>98%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.325 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.244 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>96%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.470 k-ft
$P_n$ =	0.687 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>36%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

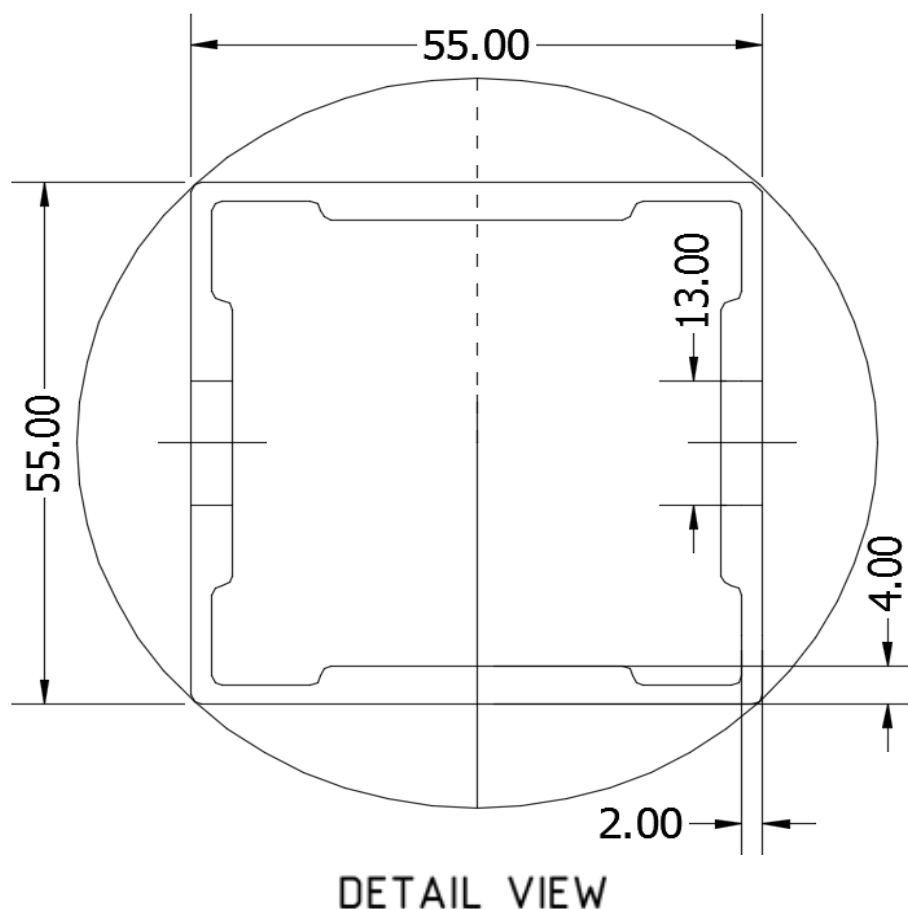
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.258 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>18%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	48.30 in
$\Phi F_{ty \text{ AXIAL}}$ =	18.93 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	-0.325 k-ft
$P_n$ =	0.696 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	18.592 k
Utilization =	<b>27%</b>



#### 5. FOUNDATION DESIGN CALCULATIONS

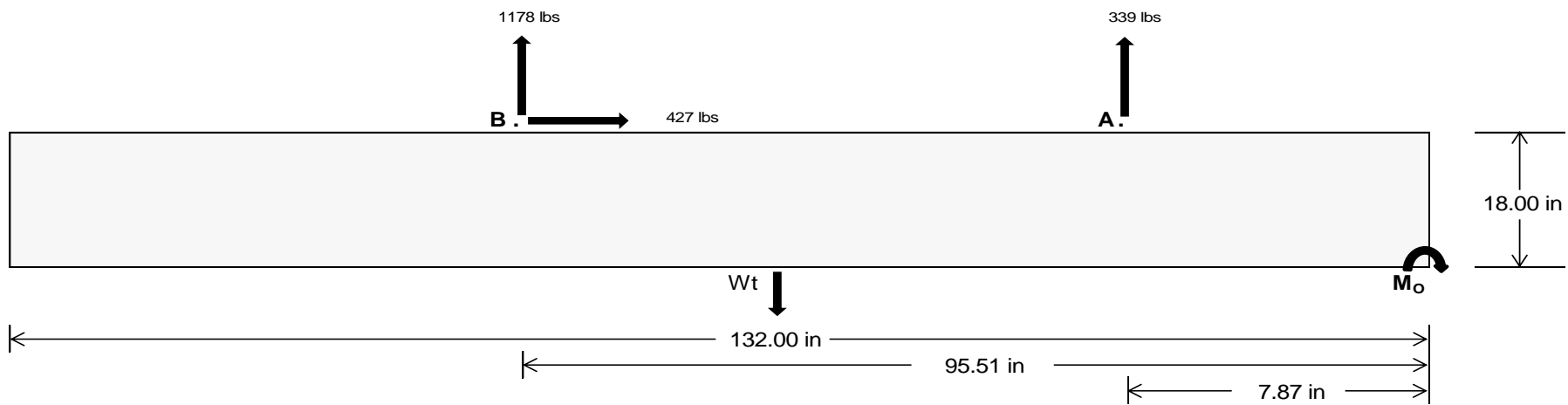
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>1490.19</b>	<b>5123.76</b> k
Compressive Load =	<b>4917.43</b>	<b>5101.97</b> k
Lateral Load =	<b>307.53</b>	<b>1853.55</b> k
Moment (Weak Axis) =	<b>0.63</b>	<b>0.42</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 122829.0$  in-lbs  
Resisting Force Required = 1861.05 lbs  
S.F. = 1.67  
Weight Required = 3101.74 lbs  
Minimum Width = 28 in in  
Weight Provided = 5582.50 lbs

### Sliding

Force = 427.45 lbs  
Friction = 0.4  
Weight Required = 1068.63 lbs  
Resisting Weight = 5582.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 427.45 lbs  
Cohesion = 130 psf  
Area = 25.67 ft<sup>2</sup>  
Resisting = 2791.25 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 28in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 28in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 28in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.33 \text{ ft}) =$

Ballast Width	28 in	29 in	30 in	31 in
	5583 lbs	5782 lbs	5981 lbs	6181 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	28 in	29 in	30 in	31 in	28 in	29 in	30 in	31 in	28 in	29 in	30 in	31 in	28 in	29 in	30 in	31 in
$F_A$	1724 lbs	1724 lbs	1724 lbs	1724 lbs	1611 lbs	1611 lbs	1611 lbs	1611 lbs	2373 lbs	2373 lbs	2373 lbs	2373 lbs	-678 lbs	-678 lbs	-678 lbs	-678 lbs
$F_B$	1786 lbs	1786 lbs	1786 lbs	1786 lbs	1670 lbs	1670 lbs	1670 lbs	1670 lbs	2460 lbs	2460 lbs	2460 lbs	2460 lbs	-2355 lbs	-2355 lbs	-2355 lbs	-2355 lbs
$F_V$	152 lbs	152 lbs	152 lbs	152 lbs	759 lbs	759 lbs	759 lbs	759 lbs	673 lbs	673 lbs	673 lbs	673 lbs	-855 lbs	-855 lbs	-855 lbs	-855 lbs
$P_{total}$	9092 lbs	9292 lbs	9491 lbs	9690 lbs	8864 lbs	9063 lbs	9262 lbs	9462 lbs	10415 lbs	10614 lbs	10814 lbs	11013 lbs	316 lbs	436 lbs	556 lbs	675 lbs
$M$	4187 lbs-ft	4187 lbs-ft	4187 lbs-ft	4187 lbs-ft	4832 lbs-ft	4832 lbs-ft	4832 lbs-ft	4832 lbs-ft	6453 lbs-ft	6453 lbs-ft	6453 lbs-ft	6453 lbs-ft	1225 lbs-ft	1225 lbs-ft	1225 lbs-ft	1225 lbs-ft
$e$	0.46 ft	0.45 ft	0.44 ft	0.43 ft	0.55 ft	0.53 ft	0.52 ft	0.51 ft	0.62 ft	0.61 ft	0.60 ft	0.59 ft	3.87 ft	2.81 ft	2.20 ft	1.81 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	265.3 psf	263.6 psf	262.1 psf	260.6 psf	242.7 psf	241.8 psf	241.0 psf	240.2 psf	268.7 psf	266.9 psf	265.2 psf	263.7 psf	0.0 psf	0.0 psf	0.0 psf	0.2 psf
$f_{max}$	443.2 psf	435.4 psf	428.2 psf	421.4 psf	448.0 psf	440.1 psf	432.6 psf	425.7 psf	542.9 psf	531.7 psf	521.2 psf	511.4 psf	55.5 psf	44.7 psf	45.0 psf	47.3 psf

Maximum Bearing Pressure = 543 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 28in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

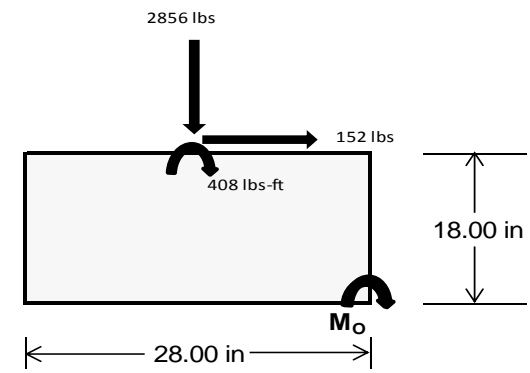
### Overturning Check

$M_o = 2695.9 \text{ ft-lbs}$   
 Resisting Force Required = 2310.78 lbs  
 S.F. = 1.67  
 Weight Required = 3851.30 lbs  
 Minimum Width = **28 in** in  
 Weight Provided = 5582.50 lbs

*A minimum 132in long x 28in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	28 in			28 in			28 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_y$	252 lbs	645 lbs	223 lbs	949 lbs	2856 lbs	927 lbs	84 lbs	189 lbs	55 lbs
$F_v$	212 lbs	208 lbs	213 lbs	158 lbs	152 lbs	165 lbs	212 lbs	209 lbs	212 lbs
$P_{total}$	7163 lbs	7556 lbs	7134 lbs	7528 lbs	9435 lbs	7506 lbs	2105 lbs	2210 lbs	2076 lbs
$M$	846 lbs-ft	840 lbs-ft	851 lbs-ft	642 lbs-ft	636 lbs-ft	665 lbs-ft	843 lbs-ft	837 lbs-ft	845 lbs-ft
$e$	0.12 ft	0.11 ft	0.12 ft	0.09 ft	0.07 ft	0.09 ft	0.40 ft	0.38 ft	0.41 ft
$L/6$	0.39 ft	0.39 ft	0.39 ft	0.39 ft	0.39 ft	0.39 ft	0.39 ft	0.39 ft	0.39 ft
$f_{min}$	194.3 psf	210.3 psf	192.7 psf	229.0 psf	303.9 psf	225.9 psf	0.0 psf	2.2 psf	0.0 psf
$f_{max}$	363.9 psf	378.5 psf	363.2 psf	357.6 psf	431.3 psf	359.0 psf	166.5 psf	169.9 psf	165.7 psf



Maximum Bearing Pressure = 431 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 28in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 28in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

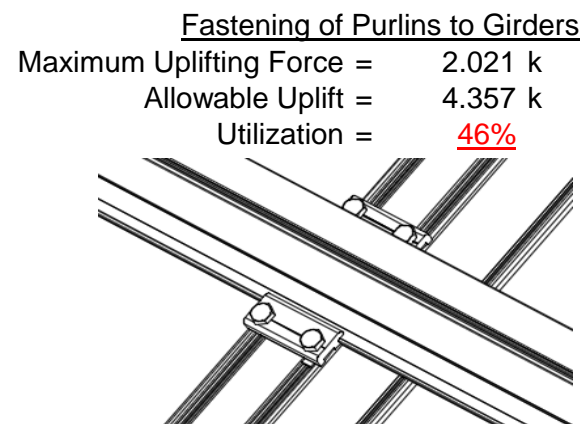
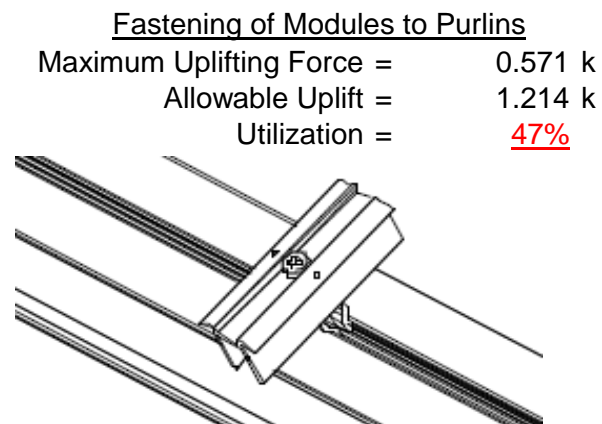
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

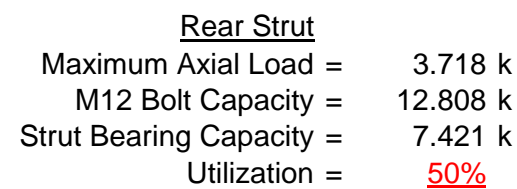
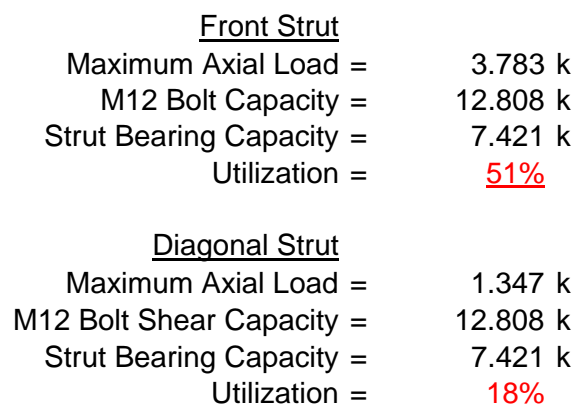
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



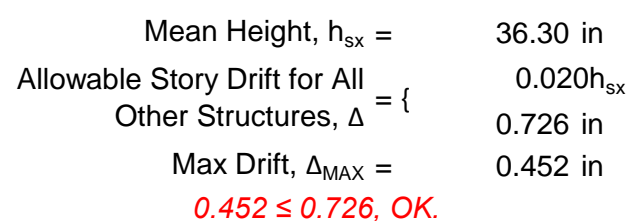
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

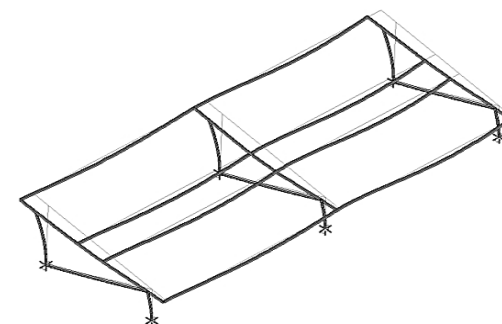
## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 120 \text{ in}$$

$$J = 0.432$$

$$331.976$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.4 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 120$$

$$J = 0.432$$

$$211.117$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-73.997	-73.997	0	0
2	M14	y	-73.997	-73.997	0	0
3	M15	y	-118.396	-118.396	0	0
4	M16	y	-118.396	-118.396	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	170.194	170.194	0	0
2	M14	y	131.716	131.716	0	0
3	M15	y	73.997	73.997	0	0
4	M16	y	73.997	73.997	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	84.096	1	898.783	1	-4.408	12	.014	1	.255	1	1.845	1
20		min	3.527	12	-1031.587	3	-147.381	1	0	3	.006	12	-2.065	3
21	11	max	84.096	1	740.052	1	-3.38	12	.014	1	.109	1	.934	1
22		min	3.527	12	-848.234	3	-115.865	1	0	3	.002	12	-1.021	3
23	12	max	84.096	1	581.321	1	-2.353	12	.014	1	.034	4	.2	1
24		min	3.527	12	-664.881	3	-84.348	1	0	3	-.002	3	-.18	3
25	13	max	84.096	1	422.59	1	-1.326	12	.014	1	.015	5	.457	3
26		min	3.527	12	-481.527	3	-52.832	1	0	3	-.078	1	-.357	1
27	14	max	84.096	1	263.859	1	-.298	12	.014	1	0	15	.89	3
28		min	.689	15	-298.174	3	-21.315	1	0	3	-.119	1	-.739	1
29	15	max	84.096	1	105.128	1	10.202	1	.014	1	-.004	12	1.119	3
30		min	-10.269	5	-114.821	3	-12.556	5	0	3	-.126	1	-.944	1
31	16	max	84.096	1	68.532	3	41.718	1	.014	1	-.003	12	1.145	3
32		min	-21.675	5	-53.603	1	-10.966	5	0	3	-.097	1	-.972	1
33	17	max	84.096	1	251.885	3	73.235	1	.014	1	0	3	.967	3
34		min	-33.08	5	-212.334	1	-9.377	5	0	3	-.048	4	-.825	1
35	18	max	84.096	1	435.239	3	104.752	1	.014	1	.066	1	.585	3
36		min	-44.486	5	-371.065	1	-7.787	5	0	3	-.05	5	-.5	1
37	19	max	84.096	1	618.592	3	136.268	1	.014	1	.2	1	0	1
38		min	-55.892	5	-529.796	1	-6.198	5	0	3	-.058	5	0	3
39	M14	1	max	61.332	4	560.874	1	-4.971	12	.008	.228	1	0	1
40		min	1.513	12	-488.541	3	-140.484	1	-.012	1	.009	12	0	3
41	2	max	49.926	4	402.143	1	-3.944	12	.008	3	.102	4	.465	3
42		min	1.513	12	-347.942	3	-108.968	1	-.012	1	.004	12	-.535	1
43	3	max	39.21	1	243.412	1	-2.917	12	.008	3	.055	5	.773	3
44		min	1.513	12	-207.342	3	-77.451	1	-.012	1	-.014	1	-.894	1
45	4	max	39.21	1	84.681	1	-1.889	12	.008	3	.029	5	.925	3
46		min	1.513	12	-66.742	3	-45.934	1	-.012	1	-.083	1	-1.076	1
47	5	max	39.21	1	73.858	3	-.862	12	.008	3	.006	5	.922	3
48		min	1.513	12	-74.05	1	-23.862	4	-.012	1	-.116	1	-1.082	1
49	6	max	39.21	1	214.458	3	17.099	1	.008	3	-.004	12	.761	3
50		min	-3.709	5	-232.781	1	-18.868	5	-.012	1	-.115	1	-.911	1
51	7	max	39.21	1	355.058	3	48.615	1	.008	3	-.003	12	.445	3
52		min	-15.115	5	-391.513	1	-17.278	5	-.012	1	-.078	1	-.565	1
53	8	max	39.21	1	495.658	3	80.132	1	.008	3	0	10	0	15
54		min	-26.52	5	-550.244	1	-15.689	5	-.012	1	-.057	4	-.049	2
55	9	max	39.21	1	636.257	3	111.649	1	.008	3	.1	1	.658	1
56		min	-37.926	5	-708.975	1	-14.099	5	-.012	1	-.071	5	-.657	3
57	10	max	60.876	4	867.706	1	-4.275	12	.012	1	.241	1	1.534	1
58		min	1.513	12	-776.857	3	-143.165	1	-.011	2	.006	12	-1.442	3
59	11	max	49.471	4	708.975	1	-3.248	12	.012	1	.103	4	.658	1
60		min	1.513	12	-636.257	3	-111.649	1	-.008	3	.002	12	-.657	3
61	12	max	39.21	1	550.244	1	-2.22	12	.012	1	.054	5	0	15
62		min	1.513	12	-495.658	3	-80.132	1	-.008	3	-.007	1	-.049	2
63	13	max	39.21	1	391.513	1	-1.193	12	.012	1	.028	5	.445	3
64		min	1.513	12	-355.058	3	-48.615	1	-.008	3	-.078	1	-.565	1
65	14	max	39.21	1	232.781	1	-.166	12	.012	1	.004	5	.761	3
66		min	1.513	12	-214.458	3	-24.419	4	-.008	3	-.115	1	-.911	1
67	15	max	39.21	1	74.05	1	14.418	1	.012	1	-.004	12	.922	3
68		min	-3.906	5	-73.858	3	-18.98	5	-.008	3	-.116	1	-1.082	1
69	16	max	39.21	1	66.742	3	45.934	1	.012	1	-.002	12	.925	3
70		min	-15.312	5	-84.681	1	-17.391	5	-.008	3	-.083	1	-1.076	1
71	17	max	39.21	1	207.342	3	77.451	1	.012	1	.001	3	.773	3
72		min	-26.718	5	-243.412	1	-15.801	5	-.008	3	-.06	4	-.894	1
73	18	max	39.21	1	347.942	3	108.968	1	.012	1	.089	1	.465	3
74		min	-38.124	5	-402.143	1	-14.212	5	-.008	3	-.073	5	-.535	1
75	19	max	39.21	1	488.541	3	140.484	1	.012	1	.228	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76	M15	1	min	-49.53	5	-560.874	1	-12.622	5	-.008	3	-.088	5	0	3
77			max	78.594	5	644.121	1	-4.941	12	.012	1	.228	1	0	2
78			min	-41.144	1	-272.322	3	-140.468	1	-.007	3	.009	12	0	3
79		2	max	67.188	5	460.724	1	-3.914	12	.012	1	.137	4	.26	3
80			min	-41.144	1	-195.855	3	-108.952	1	-.007	3	.004	12	-.614	1
81		3	max	55.782	5	277.327	1	-2.886	12	.012	1	.08	5	.435	3
82			min	-41.144	1	-119.387	3	-77.435	1	-.007	3	-.014	1	-1.024	1
83		4	max	44.376	5	93.93	1	-1.859	12	.012	1	.045	5	.525	3
84			min	-41.144	1	-42.92	3	-45.918	1	-.007	3	-.083	1	-1.23	1
85		5	max	32.971	5	33.548	3	-.832	12	.012	1	.011	5	.531	3
86			min	-41.144	1	-89.468	1	-32.786	4	-.007	3	-.116	1	-1.233	1
87		6	max	21.565	5	110.016	3	17.115	1	.012	1	-.004	12	.451	3
88	min		-41.144	1	-272.865	1	-27.789	5	-.007	3	-.115	1	-1.031	1	
89	7	max	10.159	5	186.483	3	48.632	1	.012	1	-.003	12	.286	3	
90		min	-41.144	1	-456.262	1	-26.2	5	-.007	3	-.078	1	-.626	1	
91	8	max	-.797	15	262.951	3	80.148	1	.012	1	0	10	.036	3	
92		min	-41.144	1	-639.659	1	-24.61	5	-.007	3	-.081	4	-.017	1	
93	9	max	-1.771	12	339.419	3	111.665	1	.012	1	.1	1	.795	1	
94		min	-41.144	1	-823.056	1	-23.021	5	-.007	3	-.105	5	-.298	3	
95	10	max	-1.771	12	1006.453	1	-4.305	12	.007	3	.241	1	1.812	1	
96		min	-41.144	1	-415.886	3	-143.181	1	-.012	1	.006	12	-.718	3	
97	11	max	8.315	5	823.056	1	-3.278	12	.007	3	.137	4	.795	1	
98		min	-41.144	1	-339.419	3	-111.665	1	-.012	1	.002	12	-.298	3	
99	12	max	-1.771	12	639.659	1	-2.25	12	.007	3	.078	5	.036	3	
100		min	-41.144	1	-262.951	3	-80.148	1	-.012	1	-.007	1	-.017	1	
101	13	max	-1.771	12	456.262	1	-1.223	12	.007	3	.042	5	.286	3	
102		min	-41.144	1	-186.483	3	-48.632	1	-.012	1	-.078	1	-.626	1	
103	14	max	-1.771	12	272.865	1	-.196	12	.007	3	.009	5	.451	3	
104		min	-41.144	1	-110.016	3	-33.357	4	-.012	1	-.115	1	-1.031	1	
105	15	max	-1.771	12	89.468	1	14.402	1	.007	3	-.004	12	.531	3	
106		min	-46.092	4	-33.548	3	-27.903	5	-.012	1	-.116	1	-1.233	1	
107	16	max	-1.771	12	42.92	3	45.918	1	.007	3	-.002	12	.525	3	
108		min	-57.498	4	-93.93	1	-26.314	5	-.012	1	-.083	1	-1.23	1	
109	17	max	-1.771	12	119.387	3	77.435	1	.007	3	0	3	.435	3	
110		min	-68.904	4	-277.327	1	-24.724	5	-.012	1	-.086	4	-1.024	1	
111	18	max	-1.771	12	195.855	3	108.952	1	.007	3	.089	1	.26	3	
112		min	-80.309	4	-460.724	1	-23.135	5	-.012	1	-.108	5	-.614	1	
113	19	max	-1.771	12	272.322	3	140.468	1	.007	3	.228	1	0	2	
114		min	-91.715	4	-644.121	1	-21.545	5	-.012	1	-.133	5	0	5	
115	M16	1	max	78.392	5	613.434	1	-4.739	12	.013	1	.201	1	0	1
116			min	-88.778	1	-254.138	3	-136.452	1	-.009	3	.008	12	0	3
117		2	max	66.986	5	430.037	1	-3.711	12	.013	1	.101	4	.24	3
118			min	-88.778	1	-177.67	3	-104.936	1	-.009	3	.003	12	-.58	1
119		3	max	55.58	5	246.64	1	-2.684	12	.013	1	.059	5	.395	3
120			min	-88.778	1	-101.203	3	-73.419	1	-.009	3	-.032	1	-.956	1
121		4	max	44.175	5	63.243	1	-1.656	12	.013	1	.033	5	.465	3
122			min	-88.778	1	-24.735	3	-41.902	1	-.009	3	-.096	1	-1.128	1
123		5	max	32.769	5	51.732	3	-.629	12	.013	1	.009	5	.45	3
124			min	-88.778	1	-120.154	1	-23.456	4	-.009	3	-.125	1	-1.096	1
125		6	max	21.363	5	128.2	3	21.131	1	.013	1	-.004	12	.35	3
126			min	-88.778	1	-303.552	1	-19.405	5	-.009	3	-.119	1	-.861	1
127		7	max	9.957	5	204.668	3	52.647	1	.013	1	-.003	12	.165	3
128			min	-88.778	1	-486.949	1	-17.816	5	-.009	3	-.078	1	-.422	1
129		8	max	-.912	15	281.135	3	84.164	1	.013	1	0	10	.221	1
130			min	-88.778	1	-670.346	1	-16.226	5	-.009	3	-.055	4	-.105	3
131		9	max	-3.537	12	357.603	3	115.681	1	.013	1	.109	1	1.068	1
132			min	-88.778	1	-853.743	1	-14.637	5	-.009	3	-.071	5	-.46	3





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-3.537	12	1037.14	1	-4.508	12	.013	1	.255	1	2.119	1
134			min	-88.778	1	-434.071	3	-147.197	1	-.009	3	.007	12	-.9	3
135		11	max	3.917	5	853.743	1	-3.48	12	.009	3	.109	1	1.068	1
136			min	-88.778	1	-357.603	3	-115.681	1	-.013	1	.002	12	-.46	3
137		12	max	-3.537	12	670.346	1	-2.453	12	.009	3	.053	4	.221	1
138			min	-88.778	1	-281.135	3	-84.164	1	-.013	1	-.002	1	-.105	3
139		13	max	-3.537	12	486.949	1	-1.426	12	.009	3	.027	5	.165	3
140			min	-88.778	1	-204.668	3	-52.647	1	-.013	1	-.078	1	-.422	1
141		14	max	-3.537	12	303.552	1	-.398	12	.009	3	.002	5	.35	3
142			min	-88.778	1	-128.2	3	-26.057	4	-.013	1	-.119	1	-.861	1
143		15	max	-3.537	12	120.154	1	10.386	1	.009	3	-.004	12	.45	3
144			min	-88.778	1	-51.732	3	-19.933	5	-.013	1	-.125	1	-1.096	1
145		16	max	-3.537	12	24.735	3	41.902	1	.009	3	-.003	12	.465	3
146			min	-88.778	1	-63.243	1	-18.343	5	-.013	1	-.096	1	-1.128	1
147		17	max	-3.537	12	101.203	3	73.419	1	.009	3	0	12	.395	3
148			min	-88.778	1	-246.64	1	-16.754	5	-.013	1	-.069	4	-.956	1
149		18	max	-3.537	12	177.67	3	104.936	1	.009	3	.067	1	.24	3
150			min	-94.573	4	-430.037	1	-15.164	5	-.013	1	-.08	5	-.58	1
151		19	max	-3.537	12	254.138	3	136.452	1	.009	3	.201	1	0	1
152			min	-105.979	4	-613.434	1	-13.575	5	-.013	1	-.096	5	0	5
153	M2	1	max	1153.033	1	2.333	4	1.196	1	0	3	0	3	0	1
154			min	-1094.6	3	.572	15	-78.273	4	0	4	0	1	0	1
155		2	max	1153.361	1	2.317	4	1.196	1	0	3	0	1	0	15
156			min	-1094.354	3	.568	15	-78.558	4	0	4	-.017	4	0	4
157		3	max	1153.69	1	2.302	4	1.196	1	0	3	0	1	0	15
158			min	-1094.107	3	.565	15	-78.843	4	0	4	-.035	4	-.001	4
159		4	max	1154.018	1	2.287	4	1.196	1	0	3	0	1	0	15
160			min	-1093.861	3	.561	15	-79.128	4	0	4	-.052	4	-.002	4
161		5	max	1154.346	1	2.272	4	1.196	1	0	3	.001	1	0	15
162			min	-1093.615	3	.557	15	-79.413	4	0	4	-.07	4	-.002	4
163		6	max	1154.675	1	2.256	4	1.196	1	0	3	.001	1	0	15
164			min	-1093.368	3	.554	15	-79.697	4	0	4	-.087	4	-.003	4
165		7	max	1155.003	1	2.241	4	1.196	1	0	3	.002	1	0	15
166			min	-1093.122	3	.55	15	-79.982	4	0	4	-.105	4	-.003	4
167		8	max	1155.332	1	2.226	4	1.196	1	0	3	.002	1	0	15
168			min	-1092.876	3	.547	15	-80.267	4	0	4	-.123	4	-.004	4
169		9	max	1155.66	1	2.21	4	1.196	1	0	3	.002	1	0	15
170			min	-1092.629	3	.543	15	-80.552	4	0	4	-.141	4	-.004	4
171		10	max	1155.989	1	2.195	4	1.196	1	0	3	.002	1	-.001	15
172			min	-1092.383	3	.54	15	-80.837	4	0	4	-.159	4	-.005	4
173		11	max	1156.317	1	2.18	4	1.196	1	0	3	.003	1	-.001	15
174			min	-1092.137	3	.536	15	-81.122	4	0	4	-.177	4	-.005	4
175		12	max	1156.646	1	2.165	4	1.196	1	0	3	.003	1	-.001	15
176			min	-1091.89	3	.532	15	-81.406	4	0	4	-.195	4	-.005	4
177		13	max	1156.974	1	2.149	4	1.196	1	0	3	.003	1	-.001	15
178			min	-1091.644	3	.529	15	-81.691	4	0	4	-.213	4	-.006	4
179		14	max	1157.302	1	2.134	4	1.196	1	0	3	.003	1	-.002	15
180			min	-1091.398	3	.525	15	-81.976	4	0	4	-.231	4	-.006	4
181		15	max	1157.631	1	2.119	4	1.196	1	0	3	.004	1	-.002	15
182			min	-1091.151	3	.522	15	-82.261	4	0	4	-.249	4	-.007	4
183		16	max	1157.959	1	2.104	4	1.196	1	0	3	.004	1	-.002	15
184			min	-1090.905	3	.518	15	-82.546	4	0	4	-.267	4	-.007	4
185		17	max	1158.288	1	2.088	4	1.196	1	0	3	.004	1	-.002	15
186			min	-1090.659	3	.514	15	-82.831	4	0	4	-.286	4	-.008	4
187		18	max	1158.616	1	2.073	4	1.196	1	0	3	.004	1	-.002	15
188			min	-1090.412	3	.511	15	-83.115	4	0	4	-.304	4	-.008	4
189		19	max	1158.945	1	2.058	4	1.196	1	0	3	.005	1	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1090.166	3	.507	15	-83.4	4	0	4	-.322	4	-.009	4
191	M3	1	max	312.934	2	8.105	4	.014	1	0	3	0	.009	4
192		min	-427.376	3	1.918	15	-1.199	5	0	4	-.012	4	.002	15
193		2	max	312.764	2	7.333	4	.014	1	0	3	0	.005	4
194		min	-427.504	3	1.736	15	-.657	5	0	4	-.012	4	.001	12
195		3	max	312.594	2	6.56	4	.014	1	0	3	0	.003	2
196		min	-427.631	3	1.554	15	-.115	5	0	4	-.013	4	0	3
197		4	max	312.423	2	5.788	4	.48	4	0	3	0	0	2
198		min	-427.759	3	1.373	15	0	12	0	4	-.013	4	-.001	3
199		5	max	312.253	2	5.015	4	1.022	4	0	3	0	0	15
200		min	-427.887	3	1.191	15	0	12	0	4	-.012	4	-.003	3
201		6	max	312.082	2	4.243	4	1.565	4	0	3	0	0	15
202		min	-428.015	3	1.01	15	0	12	0	4	-.012	4	-.004	6
203		7	max	311.912	2	3.471	4	2.107	4	0	3	0	1	15
204		min	-428.142	3	.828	15	0	12	0	4	-.011	4	-.006	6
205		8	max	311.742	2	2.698	4	2.649	4	0	3	0	1	15
206		min	-428.27	3	.647	15	0	12	0	4	-.01	4	-.007	6
207		9	max	311.571	2	1.926	4	3.191	4	0	3	0	1	15
208		min	-428.398	3	.465	15	0	12	0	4	-.009	4	-.008	6
209		10	max	311.401	2	1.153	4	3.733	4	0	3	0	1	15
210		min	-428.526	3	.283	15	0	12	0	4	-.007	5	-.009	6
211		11	max	311.231	2	.411	2	4.275	4	0	3	0	1	15
212		min	-428.653	3	.019	3	0	12	0	4	-.006	5	-.009	6
213		12	max	311.06	2	-.08	15	4.817	4	0	3	0	1	15
214		min	-428.781	3	-.433	3	0	12	0	4	-.004	5	-.009	6
215		13	max	310.89	2	-.261	15	5.359	4	0	3	0	1	15
216		min	-428.909	3	-1.165	6	0	12	0	4	-.002	5	-.009	6
217		14	max	310.72	2	-.443	15	5.902	4	0	3	0	4	15
218		min	-429.037	3	-1.937	6	0	12	0	4	0	12	-.008	6
219		15	max	310.549	2	-.624	15	6.444	4	0	3	.004	4	15
220		min	-429.164	3	-2.71	6	0	12	0	4	0	12	-.007	6
221		16	max	310.379	2	-.806	15	6.986	4	0	3	.006	4	15
222		min	-429.292	3	-3.482	6	0	12	0	4	0	12	-.006	6
223		17	max	310.209	2	-.988	15	7.528	4	0	3	.009	4	15
224		min	-429.42	3	-4.255	6	0	12	0	4	0	12	-.004	6
225		18	max	310.038	2	-1.169	15	8.07	4	0	3	.013	4	15
226		min	-429.548	3	-5.027	6	0	12	0	4	0	12	-.002	6
227		19	max	309.868	2	-1.351	15	8.612	4	0	3	.016	4	1
228		min	-429.676	3	-5.8	6	0	12	0	4	0	12	0	1
229	M4	1	max	1281.461	1	0	1	-.316	12	0	1	.009	4	1
230		min	-336.583	3	0	1	-235.438	4	0	1	0	10	0	1
231		2	max	1281.632	1	0	1	-.316	12	0	1	0	12	1
232		min	-336.455	3	0	1	-235.586	4	0	1	-.018	4	0	1
233		3	max	1281.802	1	0	1	-.316	12	0	1	0	12	1
234		min	-336.328	3	0	1	-235.733	4	0	1	-.046	4	0	1
235		4	max	1281.972	1	0	1	-.316	12	0	1	0	12	1
236		min	-336.2	3	0	1	-235.881	4	0	1	-.073	4	0	1
237		5	max	1282.143	1	0	1	-.316	12	0	1	0	12	1
238		min	-336.072	3	0	1	-236.029	4	0	1	-.1	4	0	1
239		6	max	1282.313	1	0	1	-.316	12	0	1	0	12	1
240		min	-335.944	3	0	1	-236.176	4	0	1	-.127	4	0	1
241		7	max	1282.484	1	0	1	-.316	12	0	1	0	12	1
242		min	-335.817	3	0	1	-236.324	4	0	1	-.154	4	0	1
243		8	max	1282.654	1	0	1	-.316	12	0	1	0	12	1
244		min	-335.689	3	0	1	-236.472	4	0	1	-.181	4	0	1
245		9	max	1282.824	1	0	1	-.316	12	0	1	0	12	1
246		min	-335.561	3	0	1	-236.619	4	0	1	-.208	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1282.995	1	0	1	-.316	12	0	1	0	12	0	1
248		min	-335.433	3	0	1	-236.767	4	0	1	-.235	4	0	1
249	11	max	1283.165	1	0	1	-.316	12	0	1	0	12	0	1
250		min	-335.305	3	0	1	-236.914	4	0	1	-.263	4	0	1
251	12	max	1283.335	1	0	1	-.316	12	0	1	0	12	0	1
252		min	-335.178	3	0	1	-237.062	4	0	1	-.29	4	0	1
253	13	max	1283.506	1	0	1	-.316	12	0	1	0	12	0	1
254		min	-335.05	3	0	1	-237.21	4	0	1	-.317	4	0	1
255	14	max	1283.676	1	0	1	-.316	12	0	1	0	12	0	1
256		min	-334.922	3	0	1	-237.357	4	0	1	-.344	4	0	1
257	15	max	1283.846	1	0	1	-.316	12	0	1	0	12	0	1
258		min	-334.794	3	0	1	-237.505	4	0	1	-.372	4	0	1
259	16	max	1284.017	1	0	1	-.316	12	0	1	0	12	0	1
260		min	-334.667	3	0	1	-237.653	4	0	1	-.399	4	0	1
261	17	max	1284.187	1	0	1	-.316	12	0	1	0	12	0	1
262		min	-334.539	3	0	1	-237.8	4	0	1	-.426	4	0	1
263	18	max	1284.357	1	0	1	-.316	12	0	1	0	12	0	1
264		min	-334.411	3	0	1	-237.948	4	0	1	-.453	4	0	1
265	19	max	1284.528	1	0	1	-.316	12	0	1	0	12	0	1
266		min	-334.283	3	0	1	-238.096	4	0	1	-.481	4	0	1
267	M6	1	max	3712.13	1	2.751	2	0	1	0	0	4	0	1
268		min	-3585.995	3	.183	3	-78.97	4	0	4	0	1	0	1
269	2	max	3712.459	1	2.739	2	0	1	0	1	0	1	0	3
270		min	-3585.749	3	.174	3	-79.255	4	0	4	-.018	4	0	2
271	3	max	3712.787	1	2.727	2	0	1	0	1	0	1	0	3
272		min	-3585.502	3	.165	3	-79.54	4	0	4	-.035	4	-.001	2
273	4	max	3713.116	1	2.715	2	0	1	0	1	0	1	0	3
274		min	-3585.256	3	.156	3	-79.825	4	0	4	-.053	4	-.002	2
275	5	max	3713.444	1	2.703	2	0	1	0	1	0	1	0	3
276		min	-3585.01	3	.147	3	-80.109	4	0	4	-.07	4	-.002	2
277	6	max	3713.773	1	2.692	2	0	1	0	1	0	1	0	3
278		min	-3584.763	3	.138	3	-80.394	4	0	4	-.088	4	-.003	2
279	7	max	3714.101	1	2.68	2	0	1	0	1	0	1	0	3
280		min	-3584.517	3	.129	3	-80.679	4	0	4	-.106	4	-.004	2
281	8	max	3714.429	1	2.668	2	0	1	0	1	0	1	0	3
282		min	-3584.271	3	.12	3	-80.964	4	0	4	-.124	4	-.004	2
283	9	max	3714.758	1	2.656	2	0	1	0	1	0	1	0	3
284		min	-3584.024	3	.111	3	-81.249	4	0	4	-.142	4	-.005	2
285	10	max	3715.086	1	2.644	2	0	1	0	1	0	1	0	3
286		min	-3583.778	3	.102	3	-81.534	4	0	4	-.16	4	-.005	2
287	11	max	3715.415	1	2.632	2	0	1	0	1	0	1	0	3
288		min	-3583.532	3	.093	3	-81.818	4	0	4	-.178	4	-.006	2
289	12	max	3715.743	1	2.62	2	0	1	0	1	0	1	0	3
290		min	-3583.285	3	.084	3	-82.103	4	0	4	-.196	4	-.007	2
291	13	max	3716.072	1	2.608	2	0	1	0	1	0	1	0	3
292		min	-3583.039	3	.076	3	-82.388	4	0	4	-.214	4	-.007	2
293	14	max	3716.4	1	2.596	2	0	1	0	1	0	1	0	3
294		min	-3582.793	3	.067	3	-82.673	4	0	4	-.233	4	-.008	2
295	15	max	3716.729	1	2.585	2	0	1	0	1	0	1	0	3
296		min	-3582.546	3	.058	3	-82.958	4	0	4	-.251	4	-.008	2
297	16	max	3717.057	1	2.573	2	0	1	0	1	0	1	0	3
298		min	-3582.3	3	.049	3	-83.243	4	0	4	-.27	4	-.009	2
299	17	max	3717.385	1	2.561	2	0	1	0	1	0	1	0	3
300		min	-3582.054	3	.04	3	-83.527	4	0	4	-.288	4	-.009	2
301	18	max	3717.714	1	2.549	2	0	1	0	1	0	1	0	3
302		min	-3581.807	3	.031	3	-83.812	4	0	4	-.307	4	-.01	2
303	19	max	3718.042	1	2.537	2	0	1	0	1	0	1	0	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-3581.561	3	.022	3	-84.097	4	0	4	-.325	4	-.011	2
305	M7	1	max	1257.795	2	8.118	6	0	1	0	0	1	.011	2
306		min	-1344.57	3	1.905	15	-1.269	5	0	4	-.012	4	0	3
307		2	max	1257.624	2	7.345	6	0	1	0	0	1	.008	2
308		min	-1344.698	3	1.723	15	-.727	5	0	4	-.013	4	-.001	3
309		3	max	1257.454	2	6.573	6	0	1	0	0	1	.005	2
310		min	-1344.826	3	1.542	15	-.185	5	0	4	-.013	4	-.003	3
311		4	max	1257.284	2	5.8	6	.402	4	0	0	1	.003	2
312		min	-1344.953	3	1.36	15	0	1	0	4	-.013	4	-.004	3
313		5	max	1257.113	2	5.028	6	.944	4	0	0	1	.001	2
314		min	-1345.081	3	1.178	15	0	1	0	4	-.012	4	-.005	3
315		6	max	1256.943	2	4.256	6	1.487	4	0	0	1	0	2
316		min	-1345.209	3	.997	15	0	1	0	4	-.012	4	-.006	3
317		7	max	1256.772	2	3.483	6	2.029	4	0	0	1	-.001	15
318		min	-1345.337	3	.815	15	0	1	0	4	-.011	4	-.007	3
319		8	max	1256.602	2	2.711	6	2.571	4	0	0	1	-.002	15
320		min	-1345.464	3	.634	15	0	1	0	4	-.01	4	-.007	4
321		9	max	1256.432	2	1.991	2	3.113	4	0	0	1	-.002	15
322		min	-1345.592	3	.378	12	0	1	0	4	-.009	4	-.008	4
323		10	max	1256.261	2	1.389	2	3.655	4	0	0	1	-.002	15
324		min	-1345.72	3	.054	3	0	1	0	4	-.008	4	-.009	4
325		11	max	1256.091	2	.787	2	4.197	4	0	0	1	-.002	15
326		min	-1345.848	3	-.398	3	0	1	0	4	-.006	4	-.009	4
327		12	max	1255.921	2	.185	2	4.739	4	0	0	1	-.002	15
328		min	-1345.975	3	-.849	3	0	1	0	4	-.004	4	-.009	4
329		13	max	1255.75	2	-.274	15	5.281	4	0	0	1	-.002	15
330		min	-1346.103	3	-1.301	3	0	1	0	4	-.002	5	-.009	4
331		14	max	1255.58	2	-.456	15	5.824	4	0	0	4	-.002	15
332		min	-1346.231	3	-1.924	4	0	1	0	4	0	1	-.008	4
333		15	max	1255.41	2	-.637	15	6.366	4	0	.003	4	-.002	15
334		min	-1346.359	3	-2.696	4	0	1	0	4	0	1	-.007	4
335		16	max	1255.239	2	-.819	15	6.908	4	0	.006	4	-.001	15
336		min	-1346.486	3	-3.469	4	0	1	0	4	0	1	-.006	4
337		17	max	1255.069	2	-.1	15	7.45	4	0	.009	4	0	15
338		min	-1346.614	3	-4.241	4	0	1	0	4	0	1	-.004	4
339		18	max	1254.899	2	-1.182	15	7.992	4	0	.012	4	0	15
340		min	-1346.742	3	-5.013	4	0	1	0	4	0	1	-.002	4
341		19	max	1254.728	2	-1.364	15	8.534	4	0	.016	4	0	1
342		min	-1346.87	3	-5.786	4	0	1	0	4	0	1	0	1
343	M8	1	max	3779.572	1	0	1	0	1	0	.008	4	0	1
344		min	-1148.603	3	0	1	-230.207	4	0	1	0	1	0	1
345		2	max	3779.742	1	0	1	0	1	0	0	1	0	1
346		min	-1148.475	3	0	1	-230.355	4	0	1	-.018	4	0	1
347		3	max	3779.913	1	0	1	0	1	0	0	1	0	1
348		min	-1148.347	3	0	1	-230.502	4	0	1	-.045	4	0	1
349		4	max	3780.083	1	0	1	0	1	0	0	1	0	1
350		min	-1148.22	3	0	1	-230.65	4	0	1	-.071	4	0	1
351		5	max	3780.253	1	0	1	0	1	0	0	1	0	1
352		min	-1148.092	3	0	1	-230.797	4	0	1	-.098	4	0	1
353		6	max	3780.424	1	0	1	0	1	0	0	1	0	1
354		min	-1147.964	3	0	1	-230.945	4	0	1	-.124	4	0	1
355		7	max	3780.594	1	0	1	0	1	0	0	1	0	1
356		min	-1147.836	3	0	1	-231.093	4	0	1	-.151	4	0	1
357		8	max	3780.764	1	0	1	0	1	0	0	1	0	1
358		min	-1147.709	3	0	1	-231.24	4	0	1	-.177	4	0	1
359		9	max	3780.935	1	0	1	0	1	0	0	1	0	1
360		min	-1147.581	3	0	1	-231.388	4	0	1	-.204	4	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3781.105	1	0	1	0	1	0	1	0	1	0	1
362			min	-1147.453	3	0	1	-231.536	4	0	1	-.23	4	0	1
363		11	max	3781.275	1	0	1	0	1	0	1	0	1	0	1
364			min	-1147.325	3	0	1	-231.683	4	0	1	-.257	4	0	1
365		12	max	3781.446	1	0	1	0	1	0	1	0	1	0	1
366			min	-1147.197	3	0	1	-231.831	4	0	1	-.284	4	0	1
367		13	max	3781.616	1	0	1	0	1	0	1	0	1	0	1
368			min	-1147.07	3	0	1	-231.979	4	0	1	-.31	4	0	1
369		14	max	3781.787	1	0	1	0	1	0	1	0	1	0	1
370			min	-1146.942	3	0	1	-232.126	4	0	1	-.337	4	0	1
371		15	max	3781.957	1	0	1	0	1	0	1	0	1	0	1
372			min	-1146.814	3	0	1	-232.274	4	0	1	-.364	4	0	1
373		16	max	3782.127	1	0	1	0	1	0	1	0	1	0	1
374			min	-1146.686	3	0	1	-232.421	4	0	1	-.39	4	0	1
375		17	max	3782.298	1	0	1	0	1	0	1	0	1	0	1
376			min	-1146.559	3	0	1	-232.569	4	0	1	-.417	4	0	1
377		18	max	3782.468	1	0	1	0	1	0	1	0	1	0	1
378			min	-1146.431	3	0	1	-232.717	4	0	1	-.444	4	0	1
379		19	max	3782.638	1	0	1	0	1	0	1	0	1	0	1
380			min	-1146.303	3	0	1	-232.864	4	0	1	-.47	4	0	1
381	M10	1	max	1153.033	1	2.229	6	-.046	12	0	1	0	1	0	1
382			min	-1094.6	3	.503	15	-78.874	4	0	5	0	3	0	1
383		2	max	1153.361	1	2.214	6	-.046	12	0	1	0	10	0	15
384			min	-1094.354	3	.499	15	-79.159	4	0	5	-.017	4	0	6
385		3	max	1153.69	1	2.199	6	-.046	12	0	1	0	12	0	15
386			min	-1094.107	3	.495	15	-79.444	4	0	5	-.035	4	0	6
387		4	max	1154.018	1	2.184	6	-.046	12	0	1	0	12	0	15
388			min	-1093.861	3	.492	15	-79.728	4	0	5	-.053	4	-.001	6
389		5	max	1154.346	1	2.168	6	-.046	12	0	1	0	12	0	15
390			min	-1093.615	3	.488	15	-80.013	4	0	5	-.07	4	-.002	6
391		6	max	1154.675	1	2.153	6	-.046	12	0	1	0	12	0	15
392			min	-1093.368	3	.485	15	-80.298	4	0	5	-.088	4	-.002	6
393		7	max	1155.003	1	2.138	6	-.046	12	0	1	0	12	0	15
394			min	-1093.122	3	.481	15	-80.583	4	0	5	-.106	4	-.003	6
395		8	max	1155.332	1	2.123	6	-.046	12	0	1	0	12	0	15
396			min	-1092.876	3	.477	15	-80.868	4	0	5	-.124	4	-.003	6
397		9	max	1155.66	1	2.107	6	-.046	12	0	1	0	12	0	15
398			min	-1092.629	3	.474	15	-81.153	4	0	5	-.142	4	-.004	6
399		10	max	1155.989	1	2.092	6	-.046	12	0	1	0	12	0	15
400			min	-1092.383	3	.47	15	-81.437	4	0	5	-.16	4	-.004	6
401		11	max	1156.317	1	2.077	6	-.046	12	0	1	0	12	-.001	15
402			min	-1092.137	3	.467	15	-81.722	4	0	5	-.178	4	-.005	6
403		12	max	1156.646	1	2.062	6	-.046	12	0	1	0	12	-.001	15
404			min	-1091.89	3	.463	15	-82.007	4	0	5	-.196	4	-.005	6
405		13	max	1156.974	1	2.046	6	-.046	12	0	1	0	12	-.001	15
406			min	-1091.644	3	.46	15	-82.292	4	0	5	-.214	4	-.006	6
407		14	max	1157.302	1	2.031	6	-.046	12	0	1	0	12	-.001	15
408			min	-1091.398	3	.456	15	-82.577	4	0	5	-.232	4	-.006	6
409		15	max	1157.631	1	2.016	6	-.046	12	0	1	0	12	-.001	15
410			min	-1091.151	3	.452	15	-82.862	4	0	5	-.251	4	-.007	6
411		16	max	1157.959	1	2.001	6	-.046	12	0	1	0	12	-.002	15
412			min	-1090.905	3	.449	15	-83.146	4	0	5	-.269	4	-.007	6
413		17	max	1158.288	1	1.985	6	-.046	12	0	1	0	12	-.002	15
414			min	-1090.659	3	.445	15	-83.431	4	0	5	-.288	4	-.007	6
415		18	max	1158.616	1	1.97	6	-.046	12	0	1	0	12	-.002	15
416			min	-1090.412	3	.442	15	-83.716	4	0	5	-.306	4	-.008	6
417		19	max	1158.945	1	1.955	6	-.046	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1090.166	3	.438	15	-84.001	4	0	5	-.325	4	-.008	6
419	M11	1	max	312.934	2	8.051	6	0	12	0	1	0	12	.008	6
420			min	-427.376	3	1.881	15	-1.2	5	0	4	-.012	4	.002	15
421		2	max	312.764	2	7.279	6	0	12	0	1	0	12	.005	6
422			min	-427.504	3	1.7	15	-.658	5	0	4	-.013	4	.001	15
423		3	max	312.594	2	6.506	6	0	12	0	1	0	12	.003	2
424			min	-427.631	3	1.518	15	-.116	5	0	4	-.013	4	0	3
425		4	max	312.423	2	5.734	6	.474	4	0	1	0	12	0	2
426			min	-427.759	3	1.337	15	-.014	1	0	4	-.013	4	-.001	3
427		5	max	312.253	2	4.961	6	1.016	4	0	1	0	12	0	15
428			min	-427.887	3	1.155	15	-.014	1	0	4	-.012	4	-.003	4
429		6	max	312.082	2	4.189	6	1.558	4	0	1	0	12	-.001	15
430			min	-428.015	3	.973	15	-.014	1	0	4	-.012	4	-.005	4
431		7	max	311.912	2	3.416	6	2.1	4	0	1	0	12	-.002	15
432			min	-428.142	3	.792	15	-.014	1	0	4	-.011	4	-.006	4
433		8	max	311.742	2	2.644	6	2.642	4	0	1	0	12	-.002	15
434			min	-428.27	3	.61	15	-.014	1	0	4	-.01	4	-.007	4
435		9	max	311.571	2	1.872	6	3.184	4	0	1	0	12	-.002	15
436			min	-428.398	3	.429	15	-.014	1	0	4	-.009	4	-.008	4
437		10	max	311.401	2	1.099	6	3.727	4	0	1	0	12	-.002	15
438			min	-428.526	3	.247	15	-.014	1	0	4	-.007	4	-.009	4
439		11	max	311.231	2	.411	2	4.269	4	0	1	0	12	-.002	15
440			min	-428.653	3	.019	3	-.014	1	0	4	-.006	4	-.009	4
441		12	max	311.06	2	-.116	15	4.811	4	0	1	0	12	-.002	15
442			min	-428.781	3	-.447	4	-.014	1	0	4	-.004	4	-.009	4
443		13	max	310.89	2	-.298	15	5.353	4	0	1	0	12	-.002	15
444			min	-428.909	3	-1.219	4	-.014	1	0	4	-.002	4	-.009	4
445		14	max	310.72	2	-.479	15	5.895	4	0	1	0	4	-.002	15
446			min	-429.037	3	-1.992	4	-.014	1	0	4	0	1	-.008	4
447		15	max	310.549	2	-.661	15	6.437	4	0	1	.003	4	-.002	15
448			min	-429.164	3	-2.764	4	-.014	1	0	4	0	1	-.007	4
449		16	max	310.379	2	-.842	15	6.979	4	0	1	.006	4	-.001	15
450			min	-429.292	3	-3.536	4	-.014	1	0	4	0	1	-.006	4
451		17	max	310.209	2	-1.024	15	7.522	4	0	1	.009	4	-.001	15
452			min	-429.42	3	-4.309	4	-.014	1	0	4	0	1	-.004	4
453		18	max	310.038	2	-1.205	15	8.064	4	0	1	.013	4	0	15
454			min	-429.548	3	-5.081	4	-.014	1	0	4	0	1	-.002	4
455		19	max	309.868	2	-1.387	15	8.606	4	0	1	.016	4	0	1
456			min	-429.676	3	-5.854	4	-.014	1	0	4	0	1	0	1
457	M12	1	max	1281.461	1	0	1	7.99	1	0	1	.009	4	0	1
458			min	-336.583	3	0	1	-231.413	4	0	1	0	1	0	1
459		2	max	1281.632	1	0	1	7.99	1	0	1	0	1	0	1
460			min	-336.455	3	0	1	-231.561	4	0	1	-.018	4	0	1
461		3	max	1281.802	1	0	1	7.99	1	0	1	.002	1	0	1
462			min	-336.328	3	0	1	-231.709	4	0	1	-.045	4	0	1
463		4	max	1281.972	1	0	1	7.99	1	0	1	.003	1	0	1
464			min	-336.2	3	0	1	-231.856	4	0	1	-.071	4	0	1
465		5	max	1282.143	1	0	1	7.99	1	0	1	.004	1	0	1
466			min	-336.072	3	0	1	-232.004	4	0	1	-.098	4	0	1
467		6	max	1282.313	1	0	1	7.99	1	0	1	.004	1	0	1
468			min	-335.944	3	0	1	-232.152	4	0	1	-.125	4	0	1
469		7	max	1282.484	1	0	1	7.99	1	0	1	.005	1	0	1
470			min	-335.817	3	0	1	-232.299	4	0	1	-.151	4	0	1
471		8	max	1282.654	1	0	1	7.99	1	0	1	.006	1	0	1
472			min	-335.689	3	0	1	-232.447	4	0	1	-.178	4	0	1
473		9	max	1282.824	1	0	1	7.99	1	0	1	.007	1	0	1
474			min	-335.561	3	0	1	-232.594	4	0	1	-.205	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1282.995	1	0	1	7.99	1	0	1	.008	1	0	1
476		min	-335.433	3	0	1	-232.742	4	0	1	-.231	4	0	1
477	11	max	1283.165	1	0	1	7.99	1	0	1	.009	1	0	1
478		min	-335.305	3	0	1	-232.89	4	0	1	-.258	4	0	1
479	12	max	1283.335	1	0	1	7.99	1	0	1	.01	1	0	1
480		min	-335.178	3	0	1	-233.037	4	0	1	-.285	4	0	1
481	13	max	1283.506	1	0	1	7.99	1	0	1	.011	1	0	1
482		min	-335.05	3	0	1	-233.185	4	0	1	-.312	4	0	1
483	14	max	1283.676	1	0	1	7.99	1	0	1	.012	1	0	1
484		min	-334.922	3	0	1	-233.333	4	0	1	-.338	4	0	1
485	15	max	1283.846	1	0	1	7.99	1	0	1	.013	1	0	1
486		min	-334.794	3	0	1	-233.48	4	0	1	-.365	4	0	1
487	16	max	1284.017	1	0	1	7.99	1	0	1	.014	1	0	1
488		min	-334.667	3	0	1	-233.628	4	0	1	-.392	4	0	1
489	17	max	1284.187	1	0	1	7.99	1	0	1	.015	1	0	1
490		min	-334.539	3	0	1	-233.776	4	0	1	-.419	4	0	1
491	18	max	1284.357	1	0	1	7.99	1	0	1	.015	1	0	1
492		min	-334.411	3	0	1	-233.923	4	0	1	-.446	4	0	1
493	19	max	1284.528	1	0	1	7.99	1	0	1	.016	1	0	1
494		min	-334.283	3	0	1	-234.071	4	0	1	-.473	4	0	1
495	M1	1	max	136.271	1	618.576	3	55.881	5	0	.2	1	0	3
496		min	-6.198	5	-528.564	1	-84.017	1	0	3	-.058	5	-.014	1
497	2	max	136.642	1	617.538	3	57.122	5	0	1	.156	1	.265	1
498		min	-6.025	5	-529.947	1	-84.017	1	0	3	-.028	5	-.325	3
499	3	max	253.564	3	590.266	1	-3.479	12	0	3	.111	1	.531	1
500		min	-158.759	2	-451.539	3	-83.032	1	0	1	.001	15	-.638	3
501	4	max	253.842	3	588.883	1	-3.479	12	0	3	.067	1	.22	1
502		min	-158.389	2	-452.577	3	-83.032	1	0	1	-.008	5	-.4	3
503	5	max	254.12	3	587.499	1	-3.479	12	0	3	.024	1	-.004	15
504		min	-158.018	2	-453.614	3	-83.032	1	0	1	-.016	5	-.161	3
505	6	max	254.398	3	586.116	1	-3.479	12	0	3	0	12	.079	3
506		min	-157.647	2	-454.652	3	-83.032	1	0	1	-.028	4	-.4	1
507	7	max	254.677	3	584.732	1	-3.479	12	0	3	-.003	12	.319	3
508		min	-157.276	2	-455.69	3	-83.032	1	0	1	-.064	1	-.709	1
509	8	max	254.955	3	583.348	1	-3.479	12	0	3	-.004	12	.56	3
510		min	-156.906	2	-456.728	3	-83.032	1	0	1	-.108	1	-1.017	1
511	9	max	262.753	3	42.048	2	38.662	5	0	9	.064	1	.654	3
512		min	-102.563	2	.417	15	-122.265	1	0	3	-.116	5	-1.159	1
513	10	max	263.031	3	40.665	2	39.903	5	0	9	0	10	.637	3
514		min	-102.192	2	0	5	-122.265	1	0	3	-.096	4	-1.17	1
515	11	max	263.309	3	39.281	2	41.145	5	0	9	-.003	12	.621	3
516		min	-101.821	2	-1.734	4	-122.265	1	0	3	-.087	4	-1.181	1
517	12	max	271.058	3	302.921	3	125.203	5	0	1	.107	1	.541	3
518		min	-60.748	10	-627.724	1	-81.154	1	0	3	-.168	5	-1.043	1
519	13	max	271.336	3	301.883	3	126.445	5	0	1	.064	1	.382	3
520		min	-60.439	10	-629.108	1	-81.154	1	0	3	-.102	5	-.711	1
521	14	max	271.614	3	300.846	3	127.686	5	0	1	.021	1	.223	3
522		min	-60.13	10	-630.491	1	-81.154	1	0	3	-.034	5	-.379	1
523	15	max	271.892	3	299.808	3	128.928	5	0	1	.033	5	.064	3
524		min	-59.821	10	-631.875	1	-81.154	1	0	3	-.022	1	-.046	1
525	16	max	272.17	3	298.77	3	130.169	5	0	1	.102	5	.293	2
526		min	-59.512	10	-633.259	1	-81.154	1	0	3	-.065	1	-.094	3
527	17	max	272.448	3	297.733	3	131.411	5	0	1	.171	5	.622	1
528		min	-59.203	10	-634.642	1	-81.154	1	0	3	-.108	1	-.251	3
529	18	max	13.402	5	615.974	1	-3.537	12	0	5	.142	5	.312	1
530		min	-136.821	1	-253.135	3	-107.263	4	0	1	-.154	1	-.125	3
531	19	max	13.575	5	614.591	1	-3.537	12	0	5	.096	5	.009	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532	M5	min	-136.45	1	-254.173	3	-106.022	4	0	1	-.201	1	-.013	1
533		max	294.757	1	2063.117	3	85.059	5	0	1	0	1	.029	1
534		min	8.816	12	-1790.323	1	0	1	0	4	-.129	4	-.001	3
535		max	295.128	1	2062.08	3	86.301	5	0	1	0	1	.974	1
536		min	9.001	12	-1791.706	1	0	1	0	4	-.084	4	-1.09	3
537		max	814.687	3	1802.526	1	13.758	4	0	4	0	1	1.876	1
538		min	-569.107	2	-1447.112	3	0	1	0	1	-.04	4	-2.135	3
539		max	814.965	3	1801.142	1	15	4	0	4	0	1	.925	1
540		min	-568.736	2	-1448.15	3	0	1	0	1	-.032	4	-1.372	3
541		max	815.243	3	1799.759	1	16.241	4	0	4	0	1	.014	9
542		min	-568.365	2	-1449.187	3	0	1	0	1	-.024	4	-.607	3
543		max	815.521	3	1798.375	1	17.483	4	0	4	0	1	.158	3
544		min	-567.994	2	-1450.225	3	0	1	0	1	-.015	5	-.974	1
545		max	815.799	3	1796.991	1	18.724	4	0	4	0	1	.923	3
546		min	-567.624	2	-1451.263	3	0	1	0	1	-.007	5	-1.922	1
547		max	816.077	3	1795.608	1	19.966	4	0	4	.005	4	1.689	3
548		min	-567.253	2	-1452.301	3	0	1	0	1	0	1	-2.87	1
549		max	829.054	3	140.034	2	123.558	4	0	1	0	1	1.944	3
550		min	-455.395	2	.418	15	0	1	0	1	-.152	4	-3.248	1
551		max	829.332	3	138.65	2	124.8	4	0	1	0	1	1.885	3
552		min	-455.025	2	0	15	0	1	0	1	-.087	5	-3.287	1
553		max	829.61	3	137.267	2	126.041	4	0	1	0	1	1.826	3
554		min	-454.654	2	-1.597	6	0	1	0	1	-.022	5	-3.325	1
555		max	842.685	3	955.784	3	170.372	4	0	1	0	1	1.603	3
556		min	-342.823	2	-1943.685	1	0	1	0	4	-.233	4	-2.963	1
557		max	842.963	3	954.747	3	171.614	4	0	1	0	1	1.099	3
558		min	-342.452	2	-1945.069	1	0	1	0	4	-.143	4	-1.937	1
559		max	843.242	3	953.709	3	172.855	4	0	1	0	1	.595	3
560		min	-342.082	2	-1946.452	1	0	1	0	4	-.052	4	-.91	1
561		max	843.52	3	952.671	3	174.096	4	0	1	.039	4	.182	2
562		min	-341.711	2	-1947.836	1	0	1	0	4	0	1	-.004	13
563		max	843.798	3	951.633	3	175.338	4	0	1	.132	4	1.146	1
564		min	-341.34	2	-1949.22	1	0	1	0	4	0	1	-.41	3
565		max	844.076	3	950.596	3	176.579	4	0	1	.225	4	2.174	1
566		min	-340.97	2	-1950.603	1	0	1	0	4	0	1	-.912	3
567		max	-9.201	12	2082.359	1	0	1	0	4	.221	4	1.124	1
568		min	-294.77	1	-867.308	3	-40.865	5	0	1	0	1	-.476	3
569		max	-9.015	12	2080.975	1	0	1	0	4	.201	4	.025	1
570		min	-294.399	1	-868.345	3	-39.624	5	0	1	0	1	-.019	3
571		max	136.271	1	618.576	3	84.017	1	0	3	-.008	12	0	3
572		min	4.838	12	-528.564	1	3.527	12	0	1	-.2	1	-.014	1
573		max	136.642	1	617.538	3	84.017	1	0	3	-.007	12	.265	1
574		min	5.024	12	-529.947	1	3.527	12	0	1	-.156	1	-.325	3
575		max	253.564	3	590.266	1	83.032	1	0	1	-.005	12	.531	1
576		min	-158.759	2	-451.539	3	-9.852	5	0	3	-.111	1	-.638	3
577		max	253.842	3	588.883	1	83.032	1	0	1	-.003	12	.22	1
578		min	-158.389	2	-452.577	3	-8.611	5	0	3	-.067	1	-.4	3
579		max	254.12	3	587.499	1	83.032	1	0	1	-.001	12	-.004	15
580		min	-158.018	2	-453.614	3	-7.37	5	0	3	-.024	4	-.161	3
581		max	254.398	3	586.116	1	83.032	1	0	1	.02	1	.079	3
582		min	-157.647	2	-454.652	3	-6.128	5	0	3	-.022	5	-.4	1
583		max	254.677	3	584.732	1	83.032	1	0	1	.064	1	.319	3
584		min	-157.276	2	-455.69	3	-4.887	5	0	3	-.025	5	-.709	1
585		max	254.955	3	583.348	1	83.032	1	0	1	.108	1	.56	3
586		min	-156.906	2	-456.728	3	-3.645	5	0	3	-.027	5	-1.017	1
587		max	262.753	3	42.048	2	122.265	1	0	3	-.003	12	.654	3
588		min	-102.563	2	.422	15	4.99	12	0	9	-.137	4	-1.159	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	263.031	3	40.665	2	122.265	1	0	3	0	1	.637	3
590		min	-102.192	2	.005	15	4.99	12	0	9	-.096	4	-1.17	1
591	11	max	263.309	3	39.281	2	122.265	1	0	3	.065	1	.621	3
592		min	-101.821	2	-1.695	6	4.99	12	0	9	-.068	5	-1.181	1
593	12	max	271.058	3	302.921	3	151.394	4	0	3	-.004	12	.541	3
594		min	-60.748	10	-627.724	1	3.22	12	0	1	-.202	4	-1.043	1
595	13	max	271.336	3	301.883	3	152.635	4	0	3	-.003	12	.382	3
596		min	-60.439	10	-629.108	1	3.22	12	0	1	-.122	4	-.711	1
597	14	max	271.614	3	300.846	3	153.877	4	0	3	0	12	.223	3
598		min	-60.13	10	-630.491	1	3.22	12	0	1	-.041	4	-.379	1
599	15	max	271.892	3	299.808	3	155.118	4	0	3	.041	4	.064	3
600		min	-59.821	10	-631.875	1	3.22	12	0	1	0	12	-.046	1
601	16	max	272.17	3	298.77	3	156.36	4	0	3	.123	4	.293	2
602		min	-59.512	10	-633.259	1	3.22	12	0	1	.003	12	-.094	3
603	17	max	272.448	3	297.733	3	157.601	4	0	3	.206	4	.622	1
604		min	-59.203	10	-634.642	1	3.22	12	0	1	.004	12	-.251	3
605	18	max	-4.924	12	615.974	1	88.854	1	0	1	.191	4	.312	1
606		min	-136.821	1	-253.135	3	-79.714	5	0	3	.006	12	-.125	3
607	19	max	-4.739	12	614.591	1	88.854	1	0	1	.201	1	.009	3
608		min	-136.45	1	-254.173	3	-78.472	5	0	3	.008	12	-.013	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.118	1	.005	3	9.392e-3	1	NC	1	NC	1
2			min	-.47	4	-.021	3	-.002	2	-1.565e-3	3	NC	1	NC	1
3		2	max	0	1	.27	3	.032	1	1.079e-2	1	NC	5	NC	2
4			min	-.47	4	-.109	1	-.013	5	-1.637e-3	3	824.323	3	7876.522	1
5		3	max	0	1	.506	3	.076	1	1.219e-2	1	NC	5	NC	3
6			min	-.47	4	-.288	1	-.015	5	-1.708e-3	3	455.751	3	3221.635	1
7		4	max	0	1	.648	3	.114	1	1.359e-2	1	NC	5	NC	3
8			min	-.47	4	-.388	1	-.011	5	-1.78e-3	3	358.762	3	2131.188	1
9		5	max	0	1	.68	3	.134	1	1.499e-2	1	NC	5	NC	3
10			min	-.47	4	-.394	1	-.002	5	-1.851e-3	3	342.464	3	1814.751	1
11		6	max	0	1	.604	3	.129	1	1.64e-2	1	NC	5	NC	3
12			min	-.47	4	-.309	1	.005	15	-1.923e-3	3	384.23	3	1879.006	1
13		7	max	0	1	.442	3	.102	1	1.78e-2	1	NC	5	NC	3
14			min	-.47	4	-.153	1	.005	10	-1.995e-3	3	518.45	3	2393.742	1
15		8	max	0	1	.237	3	.06	1	1.92e-2	1	NC	4	NC	2
16			min	-.47	4	.001	15	0	10	-2.066e-3	3	929.823	3	4140.698	1
17		9	max	0	1	.204	1	.018	14	2.06e-2	1	NC	4	NC	1
18			min	-.47	4	.005	15	-.004	10	-2.138e-3	3	2666.743	2	NC	1
19		10	max	0	1	.279	1	.014	3	2.2e-2	1	NC	3	NC	1
20			min	-.47	4	-.032	3	-.009	2	-2.21e-3	3	1487.59	1	NC	1
21		11	max	0	12	.204	1	.017	1	2.06e-2	1	NC	4	NC	1
22			min	-.47	4	.005	15	-.01	5	-2.138e-3	3	2666.743	2	NC	1
23		12	max	0	12	.237	3	.06	1	1.92e-2	1	NC	4	NC	2
24			min	-.47	4	0	15	-.01	5	-2.066e-3	3	929.823	3	4140.698	1
25		13	max	0	12	.442	3	.102	1	1.78e-2	1	NC	5	NC	3
26			min	-.47	4	-.153	1	-.004	5	-1.995e-3	3	518.45	3	2393.742	1
27		14	max	0	12	.604	3	.129	1	1.64e-2	1	NC	5	NC	3
28			min	-.47	4	-.309	1	.004	15	-1.923e-3	3	384.23	3	1879.006	1
29		15	max	0	12	.68	3	.134	1	1.499e-2	1	NC	5	NC	3
30			min	-.47	4	-.394	1	.009	10	-1.851e-3	3	342.464	3	1814.751	1
31		16	max	0	12	.648	3	.114	1	1.359e-2	1	NC	5	NC	3
32			min	-.47	4	-.388	1	.008	10	-1.78e-3	3	358.762	3	2131.188	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.506	3	.076	1	1.219e-2	1	NC	5	NC	3
34		min	-.47	4	-.288	1	.005	10	-1.708e-3	3	455.751	3	3221.635	1
35	18	max	0	12	.27	3	.032	1	1.079e-2	1	NC	5	NC	2
36		min	-.47	4	-.109	1	.001	10	-1.637e-3	3	824.323	3	7876.522	1
37	19	max	0	12	.118	1	.005	3	9.392e-3	1	NC	1	NC	1
38		min	-.47	4	-.021	3	-.002	2	-1.565e-3	3	NC	1	NC	1
39	M14	1	max	0	.193	3	.004	3	5.856e-3	1	NC	1	NC	1
40		min	-.376	4	-.38	1	-.002	2	-3.485e-3	3	NC	1	NC	1
41	2	max	0	1	.479	3	.022	1	7.03e-3	1	NC	5	NC	1
42		min	-.376	4	-.743	1	-.019	5	-4.243e-3	3	660.371	1	NC	1
43	3	max	0	1	.72	3	.061	1	8.205e-3	1	NC	15	NC	2
44		min	-.376	4	-1.056	1	-.022	5	-5.e-3	3	355.087	1	4022.126	1
45	4	max	0	1	.887	3	.098	1	9.379e-3	1	NC	15	NC	3
46		min	-.376	4	-1.282	1	-.015	5	-5.757e-3	3	266.005	1	2490.975	1
47	5	max	0	1	.964	3	.119	1	1.055e-2	1	9947.13	15	NC	3
48		min	-.376	4	-1.404	1	-.002	5	-6.514e-3	3	234.233	1	2046.065	1
49	6	max	0	1	.953	3	.118	1	1.173e-2	1	9822.69	15	NC	3
50		min	-.376	4	-1.422	1	.007	10	-7.271e-3	3	230.225	1	2070.372	1
51	7	max	0	1	.868	3	.094	1	1.29e-2	1	NC	15	NC	3
52		min	-.376	4	-1.352	1	.004	10	-8.029e-3	3	246.8	1	2594.321	1
53	8	max	0	1	.741	3	.056	1	1.408e-2	1	NC	15	NC	2
54		min	-.376	4	-1.228	1	0	10	-8.786e-3	3	282.828	1	4423.921	1
55	9	max	0	1	.618	3	.026	4	1.525e-2	1	NC	15	NC	1
56		min	-.376	4	-1.102	1	-.003	10	-9.543e-3	3	332.37	1	9349.503	4
57	10	max	0	1	.56	3	.013	3	1.643e-2	1	NC	5	NC	1
58		min	-.376	4	-1.041	1	-.008	2	-1.03e-2	3	362.797	1	NC	1
59	11	max	0	12	.618	3	.017	1	1.525e-2	1	NC	15	NC	1
60		min	-.376	4	-1.102	1	-.019	5	-9.543e-3	3	332.37	1	NC	1
61	12	max	0	12	.741	3	.056	1	1.408e-2	1	NC	15	NC	2
62		min	-.376	4	-1.228	1	-.021	5	-8.786e-3	3	282.828	1	4423.921	1
63	13	max	0	12	.868	3	.094	1	1.29e-2	1	NC	15	NC	3
64		min	-.376	4	-1.352	1	-.013	5	-8.029e-3	3	246.8	1	2594.321	1
65	14	max	0	12	.953	3	.118	1	1.173e-2	1	9822.342	15	NC	3
66		min	-.376	4	-1.422	1	0	15	-7.271e-3	3	230.225	1	2070.372	1
67	15	max	0	12	.964	3	.119	1	1.055e-2	1	9946.683	15	NC	3
68		min	-.376	4	-1.404	1	.008	10	-6.514e-3	3	234.233	1	2046.065	1
69	16	max	0	12	.887	3	.098	1	9.379e-3	1	NC	15	NC	3
70		min	-.376	4	-1.282	1	.006	10	-5.757e-3	3	266.005	1	2490.975	1
71	17	max	0	12	.72	3	.061	1	8.205e-3	1	NC	15	NC	2
72		min	-.376	4	-1.056	1	.004	10	-5.e-3	3	355.087	1	4022.126	1
73	18	max	0	12	.479	3	.026	4	7.03e-3	1	NC	5	NC	1
74		min	-.376	4	-.743	1	0	10	-4.243e-3	3	660.371	1	9056.816	4
75	19	max	0	12	.193	3	.004	3	5.856e-3	1	NC	1	NC	1
76		min	-.376	4	-.38	1	-.002	2	-3.485e-3	3	NC	1	NC	1
77	M15	1	max	0	.198	3	.004	3	2.951e-3	3	NC	1	NC	1
78		min	-.318	4	-.379	1	-.001	2	-5.963e-3	1	NC	1	NC	1
79	2	max	0	12	.382	3	.023	1	3.594e-3	3	NC	5	NC	1
80		min	-.318	4	-.782	1	-.028	5	-7.163e-3	1	595.935	1	8194.004	5
81	3	max	0	12	.542	3	.061	1	4.237e-3	3	NC	15	NC	2
82		min	-.318	4	-1.126	1	-.035	5	-8.364e-3	1	321.29	1	4010.675	1
83	4	max	0	12	.659	3	.098	1	4.88e-3	3	NC	15	NC	3
84		min	-.318	4	-1.372	1	-.025	5	-9.564e-3	1	241.787	1	2485.246	1
85	5	max	0	12	.726	3	.119	1	5.523e-3	3	9956.075	15	NC	3
86		min	-.318	4	-1.499	1	-.007	5	-1.076e-2	1	214.404	1	2041.605	1
87	6	max	0	12	.742	3	.118	1	6.166e-3	3	9833.187	15	NC	3
88		min	-.318	4	-1.506	1	.007	10	-1.197e-2	1	212.942	1	2065.449	1
89	7	max	0	12	.714	3	.094	1	6.809e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-.318	4	-1.415	1	.005	10	-1.317e-2	1	231.798	1	2586.332	1
91		8	max	0	12	.659	3	.056	1	7.452e-3	3	NC	15	NC	2
92			min	-.318	4	-1.263	1	0	10	-1.437e-2	1	271.487	1	4400.546	1
93		9	max	0	12	.601	3	.034	4	8.095e-3	3	NC	15	NC	1
94			min	-.318	4	-1.112	1	-.003	10	-1.557e-2	1	327.592	1	7021.017	4
95		10	max	0	1	.573	3	.012	3	8.738e-3	3	NC	5	NC	1
96			min	-.318	4	-1.04	1	-.007	2	-1.677e-2	1	363.2	1	NC	1
97		11	max	0	1	.601	3	.017	1	8.095e-3	3	NC	15	NC	1
98			min	-.318	4	-1.112	1	-.027	5	-1.557e-2	1	327.592	1	8824.569	5
99		12	max	0	1	.659	3	.056	1	7.452e-3	3	NC	15	NC	2
100			min	-.318	4	-1.263	1	-.032	5	-1.437e-2	1	271.487	1	4400.546	1
101		13	max	0	1	.714	3	.094	1	6.809e-3	3	NC	15	NC	3
102			min	-.318	4	-1.415	1	-.021	5	-1.317e-2	1	231.798	1	2586.332	1
103		14	max	0	1	.742	3	.118	1	6.166e-3	3	9832.93	15	NC	3
104			min	-.318	4	-1.506	1	-.002	5	-1.197e-2	1	212.942	1	2065.449	1
105		15	max	0	1	.726	3	.119	1	5.523e-3	3	9955.746	15	NC	3
106			min	-.318	4	-1.499	1	.008	10	-1.076e-2	1	214.404	1	2041.605	1
107		16	max	0	1	.659	3	.098	1	4.88e-3	3	NC	15	NC	3
108			min	-.318	4	-1.372	1	.007	10	-9.564e-3	1	241.787	1	2485.246	1
109		17	max	0	1	.542	3	.061	1	4.237e-3	3	NC	15	NC	2
110			min	-.318	4	-1.126	1	.004	10	-8.364e-3	1	321.29	1	4010.675	1
111		18	max	0	1	.382	3	.036	4	3.594e-3	3	NC	5	NC	1
112			min	-.318	4	-.782	1	0	10	-7.163e-3	1	595.935	1	6624.916	4
113		19	max	0	1	.198	3	.004	3	2.951e-3	3	NC	1	NC	1
114			min	-.318	4	-.379	1	-.001	2	-5.963e-3	1	NC	1	NC	1
115	M16	1	max	0	12	.113	1	.003	3	5.155e-3	3	NC	1	NC	1
116			min	-.141	4	-.065	3	-.001	2	-8.731e-3	1	NC	1	NC	1
117		2	max	0	12	.039	3	.032	1	6.061e-3	3	NC	5	NC	2
118			min	-.141	4	-.167	2	-.021	5	-9.973e-3	1	895.827	1	7919.939	1
119		3	max	0	12	.121	3	.076	1	6.967e-3	3	NC	5	NC	3
120			min	-.141	4	-.376	2	-.027	5	-1.121e-2	1	499.283	1	3228.722	1
121		4	max	0	12	.165	3	.114	1	7.872e-3	3	NC	5	NC	3
122			min	-.141	4	-.496	2	-.02	5	-1.246e-2	1	398.995	1	2131.751	1
123		5	max	0	12	.163	3	.134	1	8.778e-3	3	NC	5	NC	3
124			min	-.141	4	-.511	2	-.007	5	-1.37e-2	1	391.192	1	1812.093	1
125		6	max	0	12	.119	3	.13	1	9.684e-3	3	NC	5	NC	3
126			min	-.141	4	-.423	2	.005	15	-1.494e-2	1	461.732	1	1872.153	1
127		7	max	0	12	.041	3	.102	1	1.059e-2	3	NC	5	NC	3
128			min	-.141	4	-.255	2	.006	10	-1.618e-2	1	684.15	2	2375.957	1
129		8	max	0	12	.01	9	.06	1	1.15e-2	3	NC	3	NC	2
130			min	-.141	4	-.053	3	.002	10	-1.742e-2	1	1666.092	2	4070.291	1
131		9	max	0	12	.179	1	.024	4	1.24e-2	3	NC	4	NC	1
132			min	-.141	4	-.135	3	-.002	10	-1.866e-2	1	3441.982	3	9909.174	4
133		10	max	0	1	.265	1	.01	3	1.331e-2	3	NC	5	NC	1
134			min	-.141	4	-.172	3	-.007	2	-1.991e-2	1	1581.861	1	NC	1
135		11	max	0	1	.179	1	.018	1	1.24e-2	3	NC	4	NC	1
136			min	-.141	4	-.135	3	-.017	5	-1.866e-2	1	3441.982	3	NC	1
137		12	max	0	1	.01	9	.06	1	1.15e-2	3	NC	3	NC	2
138			min	-.141	4	-.053	3	-.018	5	-1.742e-2	1	1666.092	2	4070.291	1
139		13	max	0	1	.041	3	.102	1	1.059e-2	3	NC	5	NC	3
140			min	-.141	4	-.255	2	-.009	5	-1.618e-2	1	684.15	2	2375.957	1
141		14	max	0	1	.119	3	.13	1	9.684e-3	3	NC	5	NC	3
142			min	-.141	4	-.423	2	.004	15	-1.494e-2	1	461.732	1	1872.153	1
143		15	max	0	1	.163	3	.134	1	8.778e-3	3	NC	5	NC	3
144			min	-.141	4	-.511	2	.008	12	-1.37e-2	1	391.192	1	1812.093	1
145		16	max	0	1	.165	3	.114	1	7.872e-3	3	NC	5	NC	3
146			min	-.141	4	-.496	2	.007	12	-1.246e-2	1	398.995	1	2131.751	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.121	3	.076	1	6.967e-3	3	NC	5	NC	3
148			min	-.141	4	-.376	2	.005	10	-1.121e-2	1	499.283	1	3228.722	1
149		18	max	0	1	.039	3	.032	1	6.061e-3	3	NC	5	NC	2
150			min	-.141	4	-.167	2	.001	10	-9.973e-3	1	895.827	1	7552.195	4
151		19	max	0	1	.113	1	.003	3	5.155e-3	3	NC	1	NC	1
152			min	-.141	4	-.065	3	-.001	2	-8.731e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.003	2	.006	1	1.198e-3	5	NC	1	NC	2
154			min	-.005	3	-.006	3	-.445	4	-1.656e-4	1	NC	1	107.427	4
155		2	max	.005	1	.002	2	.006	1	1.281e-3	5	NC	1	NC	2
156			min	-.005	3	-.006	3	-.409	4	-1.534e-4	1	NC	1	117.054	4
157		3	max	.005	1	.002	2	.005	1	1.364e-3	5	NC	1	NC	2
158			min	-.004	3	-.005	3	-.372	4	-1.413e-4	1	NC	1	128.503	4
159		4	max	.004	1	.002	2	.005	1	1.448e-3	5	NC	1	NC	2
160			min	-.004	3	-.005	3	-.336	4	-1.291e-4	1	NC	1	142.248	4
161		5	max	.004	1	.001	2	.004	1	1.531e-3	5	NC	1	NC	1
162			min	-.004	3	-.005	3	-.301	4	-1.17e-4	1	NC	1	158.94	4
163		6	max	.004	1	0	2	.004	1	1.615e-3	5	NC	1	NC	1
164			min	-.004	3	-.005	3	-.267	4	-1.048e-4	1	NC	1	179.478	4
165		7	max	.004	1	0	2	.003	1	1.698e-3	5	NC	1	NC	1
166			min	-.003	3	-.005	3	-.233	4	-9.267e-5	1	NC	1	205.143	4
167		8	max	.003	1	0	2	.003	1	1.782e-3	4	NC	1	NC	1
168			min	-.003	3	-.004	3	-.201	4	-8.051e-5	1	NC	1	237.804	4
169		9	max	.003	1	0	15	.002	1	1.87e-3	4	NC	1	NC	1
170			min	-.003	3	-.004	3	-.171	4	-6.836e-5	1	NC	1	280.284	4
171		10	max	.003	1	0	15	.002	1	1.958e-3	4	NC	1	NC	1
172			min	-.003	3	-.004	3	-.142	4	-5.621e-5	1	NC	1	337	4
173		11	max	.002	1	0	15	.002	1	2.046e-3	4	NC	1	NC	1
174			min	-.002	3	-.004	3	-.115	4	-4.406e-5	1	NC	1	415.207	4
175		12	max	.002	1	0	15	.001	1	2.133e-3	4	NC	1	NC	1
176			min	-.002	3	-.003	3	-.091	4	-3.191e-5	1	NC	1	527.491	4
177		13	max	.002	1	0	15	0	1	2.221e-3	4	NC	1	NC	1
178			min	-.002	3	-.003	3	-.069	4	-1.976e-5	1	NC	1	697.326	4
179		14	max	.001	1	0	15	0	1	2.309e-3	4	NC	1	NC	1
180			min	-.001	3	-.002	3	-.049	4	-7.606e-6	1	NC	1	972.741	4
181		15	max	.001	1	0	15	0	1	2.397e-3	4	NC	1	NC	1
182			min	-.001	3	-.002	3	-.033	4	-1.29e-7	3	NC	1	1465.342	4
183		16	max	0	1	0	15	0	1	2.485e-3	4	NC	1	NC	1
184			min	0	3	-.002	3	-.019	4	4.508e-7	12	NC	1	2487.264	4
185		17	max	0	1	0	15	0	1	2.573e-3	4	NC	1	NC	1
186			min	0	3	-.001	3	-.009	4	9.649e-7	12	NC	1	5217.964	4
187		18	max	0	1	0	15	0	1	2.661e-3	4	NC	1	NC	1
188			min	0	3	0	6	-.003	4	1.479e-6	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.748e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.993e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-6.258e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.13e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	4.115e-6	1	NC	1	NC	1
194			min	0	2	-.001	6	0	12	-2.142e-6	5	NC	1	NC	1
195		3	max	0	3	0	15	.026	4	6.131e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	9.641e-7	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.038	4	1.226e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	1.759e-6	12	NC	1	NC	1
199		5	max	0	3	-.001	15	.05	4	1.839e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	2.554e-6	12	NC	1	9167.559	4
201		6	max	.001	3	-.002	15	.061	4	2.452e-3	4	NC	1	NC	1
202			min	0	2	-.008	6	0	12	3.349e-6	12	NC	1	8272.128	4
203		7	max	.001	3	-.002	15	.072	4	3.065e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204		min	0	2	-.01	6	0	12	4.144e-6	12	9461.486	6	7881.256	4
205	8	max	.001	3	-.002	15	.082	4	3.678e-3	4	NC	1	NC	1
206		min	-.001	2	-.011	6	0	12	4.939e-6	12	8421.457	6	7838.739	5
207	9	max	.002	3	-.003	15	.091	4	4.291e-3	4	NC	1	NC	1
208		min	-.001	2	-.012	6	0	12	5.734e-6	12	7798.864	6	8105.789	5
209	10	max	.002	3	-.003	15	.1	4	4.904e-3	4	NC	2	NC	1
210		min	-.001	2	-.012	6	0	12	6.529e-6	12	7482.542	6	8706.735	5
211	11	max	.002	3	-.003	15	.109	4	5.517e-3	4	NC	2	NC	1
212		min	-.002	2	-.013	6	0	12	7.324e-6	12	7424.855	6	9733.02	5
213	12	max	.002	3	-.003	15	.118	4	6.131e-3	4	NC	2	NC	1
214		min	-.002	2	-.012	6	0	12	8.119e-6	12	7623.242	6	NC	1
215	13	max	.003	3	-.002	15	.126	4	6.744e-3	4	NC	1	NC	1
216		min	-.002	2	-.012	6	0	12	8.914e-6	12	8121.443	6	NC	1
217	14	max	.003	3	-.002	15	.135	4	7.357e-3	4	NC	1	NC	1
218		min	-.002	2	-.01	6	0	12	9.708e-6	12	9032.967	6	NC	1
219	15	max	.003	3	-.002	15	.143	4	7.97e-3	4	NC	1	NC	1
220		min	-.002	2	-.009	1	0	12	1.05e-5	12	NC	1	NC	1
221	16	max	.003	3	-.001	15	.152	4	8.583e-3	4	NC	1	NC	1
222		min	-.002	2	-.008	1	0	12	1.13e-5	12	NC	1	NC	1
223	17	max	.003	3	0	15	.161	4	9.196e-3	4	NC	1	NC	1
224		min	-.002	2	-.006	1	0	12	1.209e-5	12	NC	1	NC	1
225	18	max	.004	3	0	15	.17	4	9.809e-3	4	NC	1	NC	1
226		min	-.003	2	-.005	1	0	12	1.289e-5	12	NC	1	NC	1
227	19	max	.004	3	0	5	.18	4	1.042e-2	4	NC	1	NC	1
228		min	-.003	2	-.003	1	0	12	1.368e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.002	2	12	-3.297e-7	12	NC	1	NC	2
230		min	0	3	-.004	3	-.18	4	-8.906e-4	4	NC	1	137.899	4
231	2	max	.003	1	.002	2	0	12	-3.297e-7	12	NC	1	NC	2
232		min	0	3	-.003	3	-.165	4	-8.906e-4	4	NC	1	150.136	4
233	3	max	.003	1	.002	2	0	12	-3.297e-7	12	NC	1	NC	2
234		min	0	3	-.003	3	-.151	4	-8.906e-4	4	NC	1	164.689	4
235	4	max	.003	1	.002	2	0	12	-3.297e-7	12	NC	1	NC	2
236		min	0	3	-.003	3	-.136	4	-8.906e-4	4	NC	1	182.161	4
237	5	max	.002	1	.002	2	0	12	-3.297e-7	12	NC	1	NC	2
238		min	0	3	-.003	3	-.122	4	-8.906e-4	4	NC	1	203.371	4
239	6	max	.002	1	.002	2	0	12	-3.297e-7	12	NC	1	NC	2
240		min	0	3	-.003	3	-.108	4	-8.906e-4	4	NC	1	229.458	4
241	7	max	.002	1	.001	2	0	12	-3.297e-7	12	NC	1	NC	2
242		min	0	3	-.002	3	-.095	4	-8.906e-4	4	NC	1	262.034	4
243	8	max	.002	1	.001	2	0	12	-3.297e-7	12	NC	1	NC	2
244		min	0	3	-.002	3	-.082	4	-8.906e-4	4	NC	1	303.454	4
245	9	max	.002	1	.001	2	0	12	-3.297e-7	12	NC	1	NC	1
246		min	0	3	-.002	3	-.069	4	-8.906e-4	4	NC	1	357.263	4
247	10	max	.002	1	.001	2	0	12	-3.297e-7	12	NC	1	NC	1
248		min	0	3	-.002	3	-.058	4	-8.906e-4	4	NC	1	429	4
249	11	max	.001	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
250		min	0	3	-.002	3	-.047	4	-8.906e-4	4	NC	1	527.734	4
251	12	max	.001	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
252		min	0	3	-.001	3	-.037	4	-8.906e-4	4	NC	1	669.148	4
253	13	max	.001	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
254		min	0	3	-.001	3	-.028	4	-8.906e-4	4	NC	1	882.36	4
255	14	max	0	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
256		min	0	3	-.001	3	-.02	4	-8.906e-4	4	NC	1	1226.618	4
257	15	max	0	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
258		min	0	3	0	3	-.013	4	-8.906e-4	4	NC	1	1838.531	4
259	16	max	0	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
260		min	0	3	0	3	-.008	4	-8.906e-4	4	NC	1	3095.838	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-8.906e-4	4	NC	1	6399.646	4
263		18	max	0	1	0	2	0	12	-3.297e-7	12	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-8.906e-4	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	-3.297e-7	12	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.906e-4	4	NC	1	NC	1
267	M6	1	max	.017	1	.012	2	0	1	1.25e-3	4	NC	3	NC	1
268			min	-.017	3	-.018	3	-.449	4	0	1	3836.626	2	106.521	4
269		2	max	.016	1	.011	2	0	1	1.332e-3	4	NC	3	NC	1
270			min	-.016	3	-.017	3	-.412	4	0	1	4206.277	2	116.068	4
271		3	max	.015	1	.01	2	0	1	1.414e-3	4	NC	3	NC	1
272			min	-.015	3	-.016	3	-.376	4	0	1	4651.307	2	127.421	4
273		4	max	.014	1	.009	2	0	1	1.496e-3	4	NC	1	NC	1
274			min	-.014	3	-.015	3	-.339	4	0	1	5193.064	2	141.052	4
275		5	max	.013	1	.008	2	0	1	1.578e-3	4	NC	1	NC	1
276			min	-.013	3	-.014	3	-.304	4	0	1	5861.27	2	157.604	4
277		6	max	.012	1	.007	2	0	1	1.661e-3	4	NC	1	NC	1
278			min	-.012	3	-.013	3	-.269	4	0	1	6698.312	2	177.972	4
279		7	max	.011	1	.006	2	0	1	1.743e-3	4	NC	1	NC	1
280			min	-.011	3	-.012	3	-.235	4	0	1	7766.348	2	203.425	4
281		8	max	.01	1	.005	2	0	1	1.825e-3	4	NC	1	NC	1
282			min	-.01	3	-.011	3	-.203	4	0	1	9159.588	2	235.817	4
283		9	max	.01	1	.004	2	0	1	1.907e-3	4	NC	1	NC	1
284			min	-.009	3	-.01	3	-.172	4	0	1	NC	1	277.946	4
285		10	max	.009	1	.004	2	0	1	1.989e-3	4	NC	1	NC	1
286			min	-.008	3	-.009	3	-.143	4	0	1	NC	1	334.198	4
287		11	max	.008	1	.003	2	0	1	2.072e-3	4	NC	1	NC	1
288			min	-.007	3	-.008	3	-.116	4	0	1	NC	1	411.765	4
289		12	max	.007	1	.002	2	0	1	2.154e-3	4	NC	1	NC	1
290			min	-.006	3	-.007	3	-.091	4	0	1	NC	1	523.138	4
291		13	max	.006	1	.001	2	0	1	2.236e-3	4	NC	1	NC	1
292			min	-.006	3	-.006	3	-.069	4	0	1	NC	1	691.603	4
293		14	max	.005	1	0	2	0	1	2.318e-3	4	NC	1	NC	1
294			min	-.005	3	-.005	3	-.05	4	0	1	NC	1	964.818	4
295		15	max	.004	1	0	2	0	1	2.4e-3	4	NC	1	NC	1
296			min	-.004	3	-.004	3	-.033	4	0	1	NC	1	1453.538	4
297		16	max	.003	1	0	2	0	1	2.483e-3	4	NC	1	NC	1
298			min	-.003	3	-.003	3	-.019	4	0	1	NC	1	2467.579	4
299		17	max	.002	1	0	2	0	1	2.565e-3	4	NC	1	NC	1
300			min	-.002	3	-.002	3	-.009	4	0	1	NC	1	5178.011	4
301		18	max	0	1	0	2	0	1	2.647e-3	4	NC	1	NC	1
302			min	0	3	-.001	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.729e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-6.065e-4	4	NC	1	NC	1
307		2	max	0	3	0	15	.013	4	0	1	NC	1	NC	1
308			min	0	2	-.002	3	0	1	-6.727e-6	5	NC	1	NC	1
309		3	max	.001	3	0	15	.026	4	5.953e-4	4	NC	1	NC	1
310			min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
311		4	max	.002	3	-.001	15	.038	4	1.196e-3	4	NC	1	NC	1
312			min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
313		5	max	.003	3	-.002	15	.049	4	1.797e-3	4	NC	1	NC	1
314			min	-.002	2	-.007	3	0	1	0	1	NC	1	8779.179	4
315		6	max	.003	3	-.002	15	.06	4	2.398e-3	4	NC	1	NC	1
316			min	-.003	2	-.009	3	0	1	0	1	NC	1	7894.734	4
317		7	max	.004	3	-.002	15	.071	4	2.999e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.004	2	-.01	4	0	1	0	1	9203.033	3	7491.682	4
319	8	max	.005	3	-.003	15	.081	4	3.6e-3	4	NC	1	NC	1
320		min	-.004	2	-.011	4	0	1	0	1	8483.723	3	7422.843	4
321	9	max	.005	3	-.003	15	.09	4	4.201e-3	4	NC	1	NC	1
322		min	-.005	2	-.012	4	0	1	0	1	7864.505	4	7641.533	4
323	10	max	.006	3	-.003	15	.099	4	4.802e-3	4	NC	1	NC	1
324		min	-.006	2	-.013	4	0	1	0	1	7541.505	4	8162.443	4
325	11	max	.007	3	-.003	15	.108	4	5.403e-3	4	NC	1	NC	1
326		min	-.006	2	-.013	4	0	1	0	1	7480.047	4	9059.926	4
327	12	max	.007	3	-.003	15	.116	4	6.004e-3	4	NC	1	NC	1
328		min	-.007	2	-.013	4	0	1	0	1	7677.068	4	NC	1
329	13	max	.008	3	-.003	15	.124	4	6.605e-3	4	NC	1	NC	1
330		min	-.007	2	-.012	4	0	1	0	1	8176.265	4	NC	1
331	14	max	.009	3	-.003	15	.133	4	7.205e-3	4	NC	1	NC	1
332		min	-.008	2	-.012	1	0	1	0	1	9091.627	4	NC	1
333	15	max	.009	3	-.002	15	.141	4	7.806e-3	4	NC	1	NC	1
334		min	-.009	2	-.011	1	0	1	0	1	NC	1	NC	1
335	16	max	.01	3	-.002	15	.149	4	8.407e-3	4	NC	1	NC	1
336		min	-.009	2	-.01	1	0	1	0	1	NC	1	NC	1
337	17	max	.01	3	-.001	15	.158	4	9.008e-3	4	NC	1	NC	1
338		min	-.01	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.011	3	0	15	.166	4	9.609e-3	4	NC	1	NC	1
340		min	-.01	2	-.009	1	0	1	0	1	NC	1	NC	1
341	19	max	.012	3	0	15	.176	4	1.021e-2	4	NC	1	NC	1
342		min	-.011	2	-.008	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.009	2	0	0	1	NC	1	NC	1
344		min	-.003	3	-.011	3	-.176	4	-9.127e-4	4	NC	1	140.94	4
345	2	max	.009	1	.009	2	0	1	0	1	NC	1	NC	1
346		min	-.003	3	-.011	3	-.162	4	-9.127e-4	4	NC	1	153.45	4
347	3	max	.008	1	.008	2	0	1	0	1	NC	1	NC	1
348		min	-.002	3	-.01	3	-.147	4	-9.127e-4	4	NC	1	168.326	4
349	4	max	.008	1	.008	2	0	1	0	1	NC	1	NC	1
350		min	-.002	3	-.01	3	-.133	4	-9.127e-4	4	NC	1	186.186	4
351	5	max	.007	1	.007	2	0	1	0	1	NC	1	NC	1
352		min	-.002	3	-.009	3	-.119	4	-9.127e-4	4	NC	1	207.868	4
353	6	max	.007	1	.007	2	0	1	0	1	NC	1	NC	1
354		min	-.002	3	-.008	3	-.106	4	-9.127e-4	4	NC	1	234.534	4
355	7	max	.006	1	.006	2	0	1	0	1	NC	1	NC	1
356		min	-.002	3	-.008	3	-.093	4	-9.127e-4	4	NC	1	267.833	4
357	8	max	.006	1	.006	2	0	1	0	1	NC	1	NC	1
358		min	-.002	3	-.007	3	-.08	4	-9.127e-4	4	NC	1	310.173	4
359	9	max	.005	1	.005	2	0	1	0	1	NC	1	NC	1
360		min	-.002	3	-.006	3	-.068	4	-9.127e-4	4	NC	1	365.176	4
361	10	max	.005	1	.005	2	0	1	0	1	NC	1	NC	1
362		min	-.001	3	-.006	3	-.057	4	-9.127e-4	4	NC	1	438.507	4
363	11	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
364		min	-.001	3	-.005	3	-.046	4	-9.127e-4	4	NC	1	539.433	4
365	12	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
366		min	-.001	3	-.004	3	-.036	4	-9.127e-4	4	NC	1	683.987	4
367	13	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.004	3	-.027	4	-9.127e-4	4	NC	1	901.934	4
369	14	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.003	3	-.02	4	-9.127e-4	4	NC	1	1253.84	4
371	15	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.003	3	-.013	4	-9.127e-4	4	NC	1	1879.347	4
373	16	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.008	4	-9.127e-4	4	NC	1	3164.592	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.001	3	-.004	4	-9.127e-4	4	NC	1	6541.835	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-9.127e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-9.127e-4	4	NC	1	NC	1
381	M10	1	max	.005	1	.003	2	0	12	1.25e-3	4	NC	1	NC	2
382			min	-.005	3	-.006	3	-.449	4	7.261e-6	12	NC	1	106.651	4
383		2	max	.005	1	.002	2	0	12	1.332e-3	4	NC	1	NC	2
384			min	-.005	3	-.006	3	-.412	4	6.747e-6	12	NC	1	116.21	4
385		3	max	.005	1	.002	2	0	12	1.414e-3	4	NC	1	NC	2
386			min	-.004	3	-.005	3	-.375	4	6.233e-6	12	NC	1	127.576	4
387		4	max	.004	1	.002	2	0	12	1.495e-3	4	NC	1	NC	2
388			min	-.004	3	-.005	3	-.339	4	5.719e-6	12	NC	1	141.224	4
389		5	max	.004	1	.001	2	0	12	1.577e-3	4	NC	1	NC	1
390			min	-.004	3	-.005	3	-.303	4	5.205e-6	12	NC	1	157.797	4
391		6	max	.004	1	0	2	0	12	1.659e-3	4	NC	1	NC	1
392			min	-.004	3	-.005	3	-.269	4	4.691e-6	12	NC	1	178.19	4
393		7	max	.004	1	0	2	0	12	1.741e-3	4	NC	1	NC	1
394			min	-.003	3	-.005	3	-.235	4	4.176e-6	12	NC	1	203.673	4
395		8	max	.003	1	0	2	0	12	1.822e-3	4	NC	1	NC	1
396			min	-.003	3	-.004	3	-.203	4	3.662e-6	12	NC	1	236.105	4
397		9	max	.003	1	0	2	0	12	1.904e-3	4	NC	1	NC	1
398			min	-.003	3	-.004	3	-.172	4	3.148e-6	12	NC	1	278.286	4
399		10	max	.003	1	0	2	0	12	1.986e-3	4	NC	1	NC	1
400			min	-.003	3	-.004	3	-.143	4	2.634e-6	12	NC	1	334.607	4
401		11	max	.002	1	0	10	0	12	2.068e-3	4	NC	1	NC	1
402			min	-.002	3	-.004	3	-.116	4	2.12e-6	12	NC	1	412.27	4
403		12	max	.002	1	0	15	0	12	2.149e-3	4	NC	1	NC	1
404			min	-.002	3	-.003	3	-.091	4	1.606e-6	12	NC	1	523.781	4
405		13	max	.002	1	0	15	0	12	2.231e-3	4	NC	1	NC	1
406			min	-.002	3	-.003	3	-.069	4	1.092e-6	12	NC	1	692.455	4
407		14	max	.001	1	0	15	0	12	2.313e-3	4	NC	1	NC	1
408			min	-.001	3	-.002	3	-.05	4	5.225e-7	10	NC	1	966.011	4
409		15	max	.001	1	0	15	0	12	2.395e-3	4	NC	1	NC	1
410			min	-.001	3	-.002	4	-.033	4	-4.545e-6	1	NC	1	1455.347	4
411		16	max	0	1	0	15	0	12	2.476e-3	4	NC	1	NC	1
412			min	0	3	-.002	4	-.019	4	-1.67e-5	1	NC	1	2470.68	4
413		17	max	0	1	0	15	0	12	2.558e-3	4	NC	1	NC	1
414			min	0	3	-.001	4	-.009	4	-2.885e-5	1	NC	1	5184.645	4
415		18	max	0	1	0	15	0	12	2.64e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.003	4	-4.1e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.722e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-5.315e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.649e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.047e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	-1.691e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-4.115e-6	1	NC	1	NC	1
423		3	max	0	3	0	15	.026	4	6.007e-4	4	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.472e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.038	4	1.203e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-4.532e-5	1	NC	1	NC	1
427		5	max	0	3	-.002	15	.049	4	1.806e-3	4	NC	1	NC	1
428			min	0	2	-.007	4	-.001	1	-6.592e-5	1	NC	1	9040.486	4
429		6	max	.001	3	-.002	15	.06	4	2.409e-3	4	NC	1	NC	1
430			min	0	2	-.009	4	-.001	1	-8.653e-5	1	NC	1	8151.138	4
431		7	max	.001	3	-.003	15	.071	4	3.011e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	0	2	-.01	4	-.002	1	-1.071e-4	1	9108.184	4	7758.718	4
433		8	max	.001	3	-.003	15	.081	4	3.614e-3	4	NC	1	NC	1
434			min	-.001	2	-.012	4	-.002	1	-1.277e-4	1	8131.233	4	7715.273	4
435		9	max	.002	3	-.003	15	.09	4	4.217e-3	4	NC	1	NC	1
436			min	-.001	2	-.013	4	-.002	1	-1.483e-4	1	7548.573	4	7977.151	4
437		10	max	.002	3	-.003	15	.099	4	4.819e-3	4	NC	2	NC	1
438			min	-.001	2	-.013	4	-.003	1	-1.689e-4	1	7257.136	4	8566.438	4
439		11	max	.002	3	-.003	15	.108	4	5.422e-3	4	NC	2	NC	1
440			min	-.002	2	-.013	4	-.003	1	-1.895e-4	1	7213.412	4	9572.199	4
441		12	max	.002	3	-.003	15	.116	4	6.025e-3	4	NC	2	NC	1
442			min	-.002	2	-.013	4	-.003	1	-2.101e-4	1	7416.666	4	NC	1
443		13	max	.003	3	-.003	15	.124	4	6.627e-3	4	NC	1	NC	1
444			min	-.002	2	-.012	4	-.004	1	-2.308e-4	1	7910.728	4	NC	1
445		14	max	.003	3	-.003	15	.133	4	7.23e-3	4	NC	1	NC	1
446			min	-.002	2	-.011	4	-.004	1	-2.514e-4	1	8807.225	4	NC	1
447		15	max	.003	3	-.002	15	.141	4	7.833e-3	4	NC	1	NC	1
448			min	-.002	2	-.01	4	-.004	1	-2.72e-4	1	NC	1	NC	1
449		16	max	.003	3	-.002	15	.149	4	8.435e-3	4	NC	1	NC	1
450			min	-.002	2	-.008	4	-.005	1	-2.926e-4	1	NC	1	NC	1
451		17	max	.003	3	-.001	15	.158	4	9.038e-3	4	NC	1	NC	1
452			min	-.002	2	-.006	1	-.005	1	-3.132e-4	1	NC	1	NC	1
453		18	max	.004	3	0	15	.167	4	9.641e-3	4	NC	1	NC	1
454			min	-.003	2	-.005	1	-.006	1	-3.338e-4	1	NC	1	NC	1
455		19	max	.004	3	0	12	.177	4	1.024e-2	4	NC	1	NC	1
456			min	-.003	2	-.003	1	-.006	1	-3.544e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.002	2	.006	1	1.214e-5	1	NC	1	NC	2
458			min	0	3	-.004	3	-.177	4	-8.843e-4	4	NC	1	140.321	4
459		2	max	.003	1	.002	2	.006	1	1.214e-5	1	NC	1	NC	2
460			min	0	3	-.003	3	-.162	4	-8.843e-4	4	NC	1	152.773	4
461		3	max	.003	1	.002	2	.005	1	1.214e-5	1	NC	1	NC	2
462			min	0	3	-.003	3	-.148	4	-8.843e-4	4	NC	1	167.58	4
463		4	max	.003	1	.002	2	.005	1	1.214e-5	1	NC	1	NC	2
464			min	0	3	-.003	3	-.134	4	-8.843e-4	4	NC	1	185.357	4
465		5	max	.002	1	.002	2	.004	1	1.214e-5	1	NC	1	NC	2
466			min	0	3	-.003	3	-.12	4	-8.843e-4	4	NC	1	206.939	4
467		6	max	.002	1	.002	2	.004	1	1.214e-5	1	NC	1	NC	2
468			min	0	3	-.003	3	-.106	4	-8.843e-4	4	NC	1	233.481	4
469		7	max	.002	1	.001	2	.003	1	1.214e-5	1	NC	1	NC	2
470			min	0	3	-.002	3	-.093	4	-8.843e-4	4	NC	1	266.627	4
471		8	max	.002	1	.001	2	.003	1	1.214e-5	1	NC	1	NC	2
472			min	0	3	-.002	3	-.08	4	-8.843e-4	4	NC	1	308.771	4
473		9	max	.002	1	.001	2	.002	1	1.214e-5	1	NC	1	NC	1
474			min	0	3	-.002	3	-.068	4	-8.843e-4	4	NC	1	363.52	4
475		10	max	.002	1	.001	2	.002	1	1.214e-5	1	NC	1	NC	1
476			min	0	3	-.002	3	-.057	4	-8.843e-4	4	NC	1	436.511	4
477		11	max	.001	1	0	2	.002	1	1.214e-5	1	NC	1	NC	1
478			min	0	3	-.002	3	-.046	4	-8.843e-4	4	NC	1	536.97	4
479		12	max	.001	1	0	2	.001	1	1.214e-5	1	NC	1	NC	1
480			min	0	3	-.001	3	-.036	4	-8.843e-4	4	NC	1	680.854	4
481		13	max	.001	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
482			min	0	3	-.001	3	-.028	4	-8.843e-4	4	NC	1	897.791	4
483		14	max	0	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
484			min	0	3	-.001	3	-.02	4	-8.843e-4	4	NC	1	1248.063	4
485		15	max	0	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
486			min	0	3	0	3	-.013	4	-8.843e-4	4	NC	1	1870.663	4
487		16	max	0	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
488			min	0	3	0	3	-.008	4	-8.843e-4	4	NC	1	3149.924	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-8.843e-4	4	NC	1	6511.405	4
491		18	max	0	1	0	2	0	1	1.214e-5	1	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-8.843e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	1.214e-5	1	NC	1	NC	1
494			min	0	1	0	1	0	1	-8.843e-4	4	NC	1	NC	1
495	M1	1	max	.005	3	.118	1	.47	4	1.772e-2	1	NC	1	NC	1
496			min	-.002	2	-.021	3	0	12	-2.266e-2	3	NC	1	NC	1
497		2	max	.005	3	.058	1	.458	4	8.641e-3	1	NC	5	NC	1
498			min	-.002	2	-.01	3	-.004	1	-1.121e-2	3	1915.165	1	NC	1
499		3	max	.005	3	.007	3	.445	4	1.365e-2	4	NC	5	NC	1
500			min	-.002	2	-.007	1	-.006	1	-1.143e-4	1	916.616	1	8837.157	5
501		4	max	.004	3	.036	3	.433	4	1.207e-2	4	NC	5	NC	1
502			min	-.002	2	-.082	1	-.006	1	-4.176e-3	3	572.991	1	5994.783	5
503		5	max	.004	3	.072	3	.421	4	1.048e-2	4	NC	15	NC	1
504			min	-.002	2	-.161	1	-.004	1	-8.243e-3	3	410.148	1	4565.496	5
505		6	max	.004	3	.113	3	.409	4	1.551e-2	1	NC	15	NC	1
506			min	-.002	2	-.238	1	-.002	1	-1.231e-2	3	320.997	1	3728.587	5
507		7	max	.004	3	.151	3	.396	4	2.071e-2	1	9699.005	15	NC	1
508			min	-.002	2	-.308	1	0	12	-1.638e-2	3	268.643	1	3191.558	4
509		8	max	.004	3	.183	3	.382	4	2.592e-2	1	8617.861	15	NC	1
510			min	-.002	2	-.363	1	0	12	-2.044e-2	3	237.793	1	2831.423	4
511		9	max	.004	3	.204	3	.367	4	2.857e-2	1	8054.174	15	NC	1
512			min	-.002	2	-.398	1	0	1	-2.059e-2	3	221.776	1	2629.165	4
513		10	max	.004	3	.212	3	.35	4	2.953e-2	1	7882.513	15	NC	1
514			min	-.002	2	-.41	1	0	12	-1.813e-2	3	216.984	1	2574.747	4
515		11	max	.004	3	.207	3	.33	4	3.048e-2	1	8054.014	15	NC	1
516			min	-.001	2	-.398	1	0	12	-1.567e-2	3	222.07	1	2643.449	4
517		12	max	.004	3	.189	3	.309	4	2.88e-2	1	8617.493	15	NC	1
518			min	-.001	2	-.362	1	0	1	-1.315e-2	3	238.705	1	2852.969	5
519		13	max	.004	3	.161	3	.284	4	2.313e-2	1	9698.299	15	NC	1
520			min	-.001	2	-.306	1	0	1	-1.052e-2	3	270.89	1	3365.743	4
521		14	max	.004	3	.125	3	.258	4	1.747e-2	1	NC	15	NC	1
522			min	-.001	2	-.235	1	0	12	-7.903e-3	3	325.827	1	4402.362	4
523		15	max	.004	3	.084	3	.231	4	1.181e-2	1	NC	15	NC	1
524			min	-.001	2	-.157	1	0	12	-5.282e-3	3	420.119	1	6601.331	4
525		16	max	.003	3	.042	3	.205	4	9.064e-3	4	NC	5	NC	1
526			min	-.001	2	-.078	1	0	12	-2.66e-3	3	594.044	1	NC	1
527		17	max	.003	3	.003	3	.18	4	9.968e-3	4	NC	5	NC	1
528			min	-.001	2	-.004	1	0	12	-3.907e-5	3	964.355	1	NC	1
529		18	max	.003	3	.058	1	.159	4	1.051e-2	1	NC	5	NC	1
530			min	-.001	2	-.033	3	0	12	-4.031e-3	3	2036.616	1	NC	1
531		19	max	.003	3	.113	1	.141	4	2.091e-2	1	NC	1	NC	1
532			min	-.001	2	-.065	3	0	1	-8.178e-3	3	NC	1	NC	1
533	M5	1	max	.014	3	.279	1	.47	4	0	1	NC	1	NC	1
534			min	-.009	2	-.032	3	0	1	-2.235e-6	4	NC	1	NC	1
535		2	max	.014	3	.138	1	.46	4	6.989e-3	4	NC	5	NC	1
536			min	-.009	2	-.016	3	0	1	0	1	812.213	1	NC	1
537		3	max	.014	3	.022	3	.449	4	1.376e-2	4	NC	15	NC	1
538			min	-.009	2	-.022	1	0	1	0	1	379.678	1	7398.69	4
539		4	max	.014	3	.099	3	.437	4	1.121e-2	4	9361.541	15	NC	1
540			min	-.009	2	-.218	1	0	1	0	1	230.384	1	5354.249	4
541		5	max	.014	3	.204	3	.423	4	8.664e-3	4	6551.261	15	NC	1
542			min	-.009	2	-.431	1	0	1	0	1	161.039	1	4304.02	4
543		6	max	.013	3	.321	3	.41	4	6.114e-3	4	5043.914	15	NC	1
544			min	-.008	2	-.645	1	0	1	0	1	123.848	1	3657.117	4
545		7	max	.013	3	.435	3	.396	4	3.565e-3	4	4173.402	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.008	2	-.839	1	0	1	0	1	102.369	1	3207.853	4
547		8	max	.013	3	.531	3	.382	4	1.016e-3	4	3667.235	15	NC	1
548			min	-.008	2	-.994	1	0	1	0	1	89.882	1	2865.099	4
549		9	max	.013	3	.592	3	.367	4	0	1	3407.654	15	NC	1
550			min	-.008	2	-1.092	1	0	1	-1.283e-6	5	83.482	1	2629.653	4
551		10	max	.012	3	.615	3	.35	4	0	1	3329.438	15	NC	1
552			min	-.008	2	-1.125	1	0	1	-1.214e-6	5	81.578	1	2593.273	4
553		11	max	.012	3	.599	3	.33	4	0	1	3407.712	15	NC	1
554			min	-.008	2	-1.092	1	0	1	-1.145e-6	5	83.6	1	2669.594	4
555		12	max	.012	3	.547	3	.31	4	7.178e-4	4	3667.371	15	NC	1
556			min	-.007	2	-.992	1	0	1	0	1	90.271	1	2815.106	4
557		13	max	.012	3	.463	3	.285	4	2.519e-3	4	4173.682	15	NC	1
558			min	-.007	2	-.833	1	0	1	0	1	103.38	1	3314.318	4
559		14	max	.011	3	.357	3	.258	4	4.32e-3	4	5044.467	15	NC	1
560			min	-.007	2	-.635	1	0	1	0	1	126.121	1	4518.845	4
561		15	max	.011	3	.239	3	.229	4	6.121e-3	4	6552.36	15	NC	1
562			min	-.007	2	-.419	1	0	1	0	1	165.969	1	7622.971	5
563		16	max	.011	3	.12	3	.201	4	7.922e-3	4	9363.847	15	NC	1
564			min	-.007	2	-.205	1	0	1	0	1	241.426	1	NC	1
565		17	max	.01	3	.008	3	.177	4	9.723e-3	4	NC	15	NC	1
566			min	-.007	2	-.014	1	0	1	0	1	406.532	1	NC	1
567		18	max	.01	3	.137	1	.156	4	4.938e-3	4	NC	5	NC	1
568			min	-.007	2	-.087	3	0	1	0	1	884.164	1	NC	1
569		19	max	.01	3	.265	1	.141	4	0	1	NC	1	NC	1
570			min	-.007	2	-.172	3	0	1	-9.198e-7	4	NC	1	NC	1
571	M9	1	max	.005	3	.118	1	.47	4	2.266e-2	3	NC	1	NC	1
572			min	-.002	2	-.021	3	0	1	-1.772e-2	1	NC	1	NC	1
573		2	max	.005	3	.058	1	.46	4	1.121e-2	3	NC	5	NC	1
574			min	-.002	2	-.01	3	0	12	-8.641e-3	1	1915.165	1	NC	1
575		3	max	.005	3	.007	3	.448	4	1.372e-2	4	NC	5	NC	1
576			min	-.002	2	-.007	1	0	12	-2.721e-5	10	916.616	1	7535.365	4
577		4	max	.004	3	.036	3	.436	4	1.077e-2	5	NC	5	NC	1
578			min	-.002	2	-.082	1	0	12	-5.093e-3	1	572.991	1	5403.604	4
579		5	max	.004	3	.072	3	.423	4	8.243e-3	3	NC	15	NC	1
580			min	-.002	2	-.161	1	0	12	-1.03e-2	1	410.148	1	4309.8	4
581		6	max	.004	3	.113	3	.41	4	1.231e-2	3	NC	15	NC	1
582			min	-.002	2	-.238	1	0	12	-1.551e-2	1	320.997	1	3642.25	4
583		7	max	.004	3	.151	3	.396	4	1.638e-2	3	9686.864	15	NC	1
584			min	-.002	2	-.308	1	0	1	-2.071e-2	1	268.643	1	3187.779	4
585		8	max	.004	3	.183	3	.382	4	2.044e-2	3	8607.308	15	NC	1
586			min	-.002	2	-.363	1	0	1	-2.592e-2	1	237.793	1	2849.515	5
587		9	max	.004	3	.204	3	.367	4	2.059e-2	3	8044.43	15	NC	1
588			min	-.002	2	-.398	1	0	12	-2.857e-2	1	221.776	1	2623.328	4
589		10	max	.004	3	.212	3	.35	4	1.813e-2	3	7873.004	15	NC	1
590			min	-.002	2	-.41	1	0	1	-2.953e-2	1	216.984	1	2575.593	4
591		11	max	.004	3	.207	3	.33	4	1.567e-2	3	8044.273	15	NC	1
592			min	-.001	2	-.398	1	0	1	-3.048e-2	1	222.07	1	2650.697	4
593		12	max	.004	3	.189	3	.309	4	1.315e-2	3	8607.008	15	NC	1
594			min	-.001	2	-.362	1	0	12	-2.88e-2	1	238.705	1	2834.915	4
595		13	max	.004	3	.161	3	.284	4	1.052e-2	3	9686.389	15	NC	1
596			min	-.001	2	-.306	1	0	12	-2.313e-2	1	270.89	1	3366.145	4
597		14	max	.004	3	.125	3	.257	4	7.903e-3	3	NC	15	NC	1
598			min	-.001	2	-.235	1	-.002	1	-1.747e-2	1	325.827	1	4502.467	5
599		15	max	.004	3	.084	3	.229	4	5.714e-3	5	NC	15	NC	1
600			min	-.001	2	-.157	1	-.004	1	-1.181e-2	1	420.119	1	7113.28	5
601		16	max	.003	3	.042	3	.202	4	7.698e-3	5	NC	5	NC	1
602			min	-.001	2	-.078	1	-.006	1	-6.142e-3	1	594.044	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.003	3	.003	3	.177	4	9.75e-3	4	NC	5	NC	1
604		min	-.001	2	-.004	1	-.006	1	-4.784e-4	1	964.355	1	NC	1
605	18	max	.003	3	.058	1	.157	4	4.589e-3	5	NC	5	NC	1
606		min	-.001	2	-.033	3	-.004	1	-1.051e-2	1	2036.616	1	NC	1
607	19	max	.003	3	.113	1	.141	4	8.178e-3	3	NC	1	NC	1
608		min	-.001	2	-.065	3	0	12	-2.091e-2	1	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

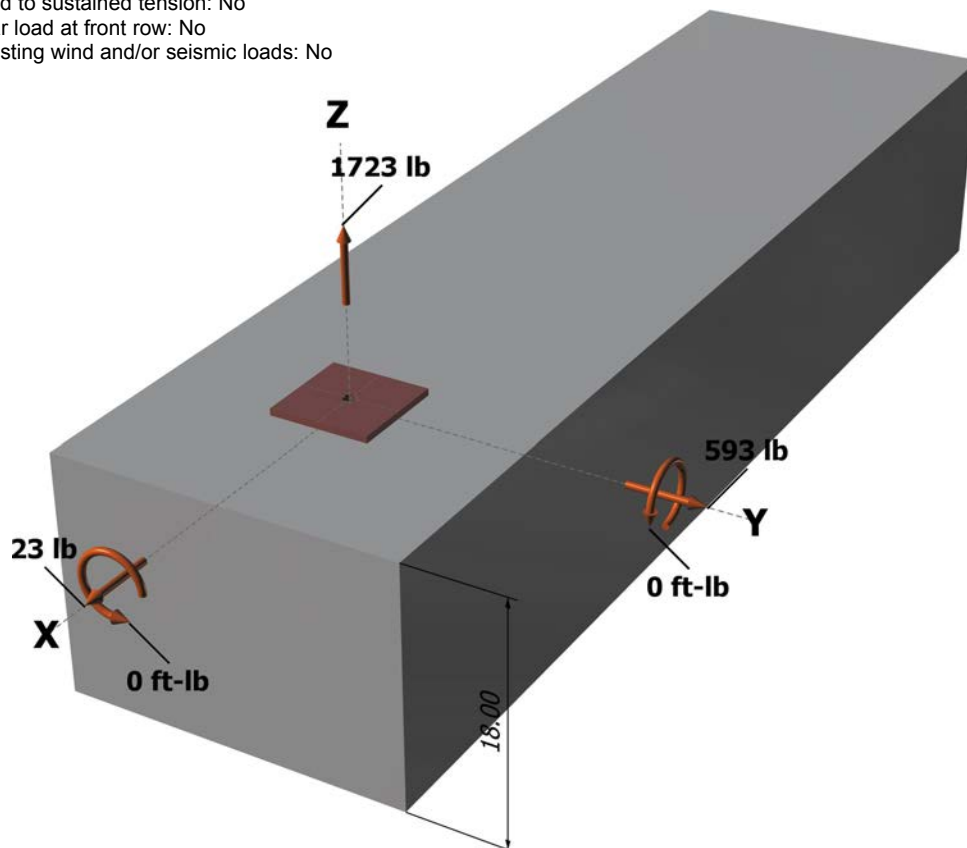
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

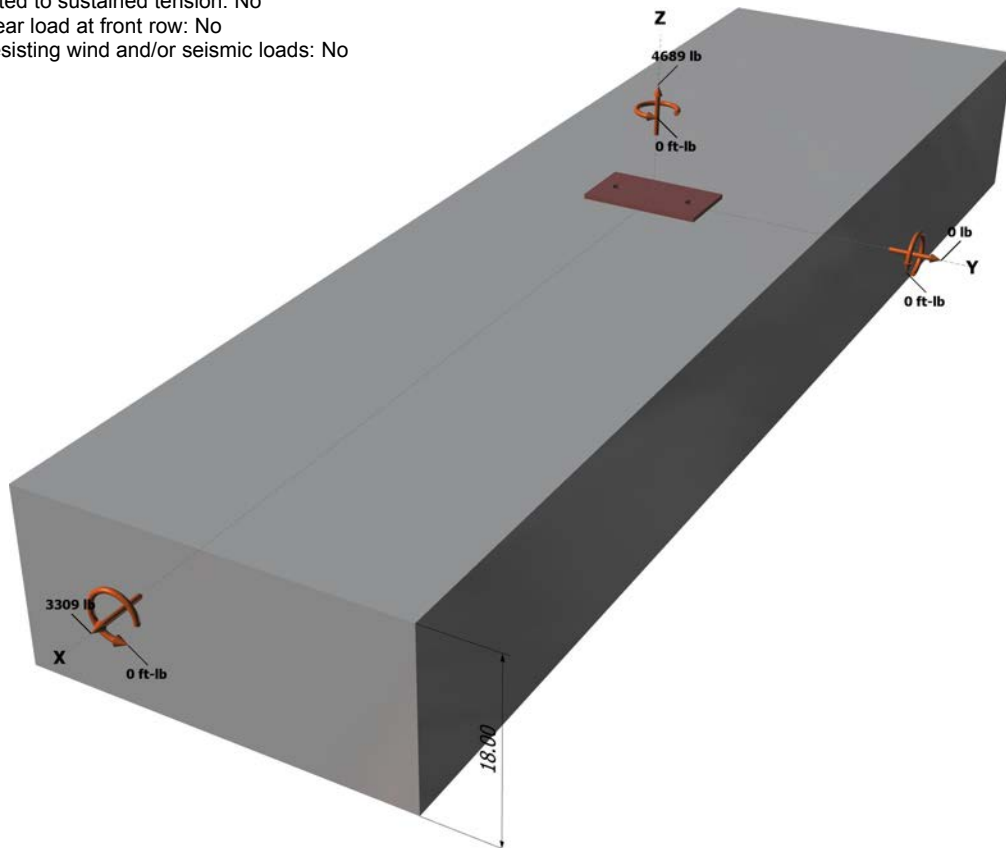
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

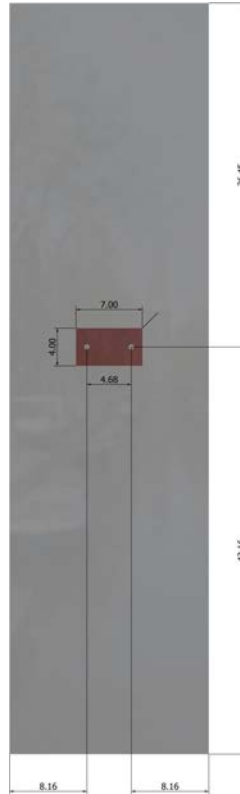
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Anchor Designer™  
Software  
Version 2.4.5673.0

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Address:			
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E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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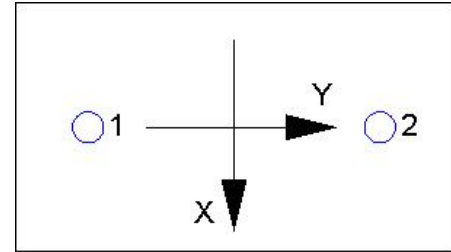
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpg} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.