



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	85 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 11.34 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	111 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.759 k-ft
$M_z$ =	0.308 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>90%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.332 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.360 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.255 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>12%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.349 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>23%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.432 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<b>26%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

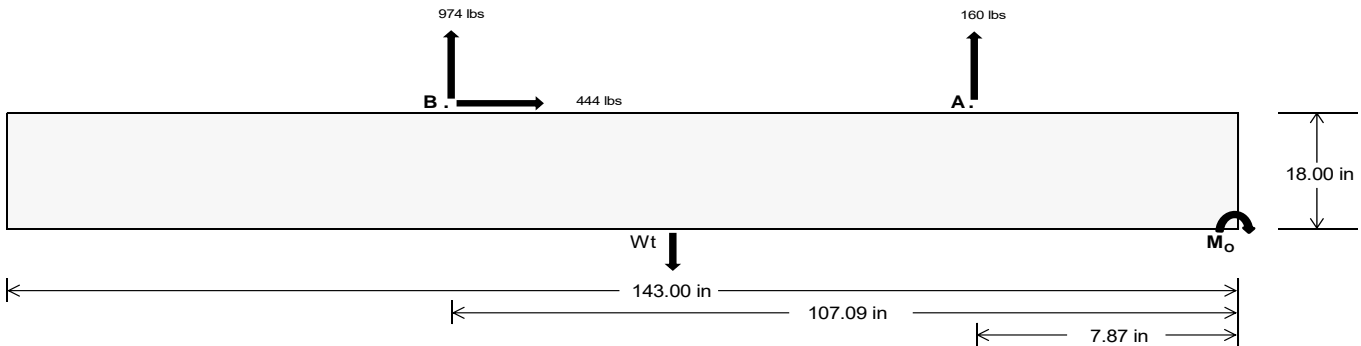
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>680.77</b>	<b>4067.38</b>	k
Compressive Load =	<b>4231.00</b>	<b>4743.69</b>	k
Lateral Load =	<b>16.94</b>	<b>1849.38</b>	k
Moment (Weak Axis) =	<b>0.04</b>	<b>0.01</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 113613.8$  in-lbs  
Resisting Force Required = 1589.00 lbs  
S.F. = 1.67  
Weight Required = 2648.34 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 444.36 lbs  
Friction = 0.4  
Weight Required = 1110.90 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 444.36 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$  35 in 36 in 37 in 38 in  
7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1627 lbs	1627 lbs	1627 lbs	1627 lbs	1216 lbs	1216 lbs	1216 lbs	1216 lbs	1999 lbs	1999 lbs	1999 lbs	1999 lbs	-321 lbs	-321 lbs	-321 lbs	-321 lbs
$F_B$	1753 lbs	1753 lbs	1753 lbs	1753 lbs	1478 lbs	1478 lbs	1478 lbs	1478 lbs	2281 lbs	2281 lbs	2281 lbs	2281 lbs	-1949 lbs	-1949 lbs	-1949 lbs	-1949 lbs
$F_V$	177 lbs	177 lbs	177 lbs	177 lbs	804 lbs	804 lbs	804 lbs	804 lbs	723 lbs	723 lbs	723 lbs	723 lbs	-889 lbs	-889 lbs	-889 lbs	-889 lbs
$P_{total}$	10939 lbs	11155 lbs	11371 lbs	11587 lbs	10253 lbs	10469 lbs	10685 lbs	10901 lbs	11840 lbs	12056 lbs	12272 lbs	12488 lbs	2266 lbs	2396 lbs	2525 lbs	2655 lbs
$M$	3692 lbs-ft	3692 lbs-ft	3692 lbs-ft	3692 lbs-ft	3269 lbs-ft	3269 lbs-ft	3269 lbs-ft	3269 lbs-ft	4919 lbs-ft	4919 lbs-ft	4919 lbs-ft	4919 lbs-ft	2745 lbs-ft	2745 lbs-ft	2745 lbs-ft	2745 lbs-ft
$e$	0.34 ft	0.33 ft	0.32 ft	0.32 ft	0.31 ft	0.31 ft	0.31 ft	0.30 ft	0.42 ft	0.41 ft	0.40 ft	0.39 ft	1.21 ft	1.15 ft	1.09 ft	1.03 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	261.3 psf	260.0 psf	258.9 psf	257.8 psf	247.6 psf	246.8 psf	246.0 psf	245.3 psf	269.4 psf	267.9 psf	266.6 psf	265.3 psf	25.4 psf	28.3 psf	31.1 psf	33.7 psf
$f_{max}$	368.2 psf	364.0 psf	360.1 psf	356.3 psf	342.4 psf	338.9 psf	335.6 psf	332.5 psf	411.9 psf	406.5 psf	401.4 psf	396.6 psf	105.0 psf	105.7 psf	106.3 psf	107.0 psf

Maximum Bearing Pressure = 412 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

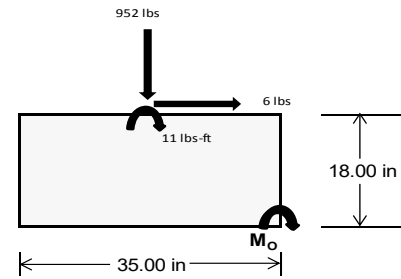
#### Overturning Check

$M_o = 1368.3 \text{ ft-lbs}$   
 Resisting Force Required = 938.27 lbs  
 S.F. = 1.67  
 Weight Required = 1563.79 lbs  
 Minimum Width = **35 in**  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	262 lbs	681 lbs	262 lbs	952 lbs	2771 lbs	952 lbs	77 lbs	199 lbs	77 lbs
$F_v$	2 lbs	0 lbs	2 lbs	6 lbs	0 lbs	6 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9621 lbs	7560 lbs	9621 lbs	9862 lbs	7560 lbs	9862 lbs	2813 lbs	7560 lbs	2813 lbs
$M$	5 lbs-ft	0 lbs-ft	5 lbs-ft	21 lbs-ft	0 lbs-ft	21 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	276.5 psf	217.5 psf	276.5 psf	282.5 psf	217.5 psf	282.5 psf	80.9 psf	217.5 psf	80.9 psf
$f_{max}$	277.1 psf	217.5 psf	277.1 psf	285.0 psf	217.5 psf	285.0 psf	81.0 psf	217.5 psf	81.0 psf



Maximum Bearing Pressure = 285 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 143in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

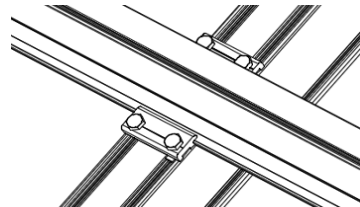
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.475 k
Allowable Uplift =	1.214 k
Utilization =	<u>39%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.540 k
Allowable Uplift =	4.357 k
Utilization =	<u>35%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.255 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

#### Rear Strut

Maximum Axial Load =	3.432 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>46%</u>

#### Diagonal Strut

Maximum Axial Load =	1.420 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>19%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 111 \text{ in}$$

$$J = 0.432$$

$$307.078$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 111$$

$$J = 0.432$$

$$195.283$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.8$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.41345$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77788$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 13.6277 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 13.63 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 14.03 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-39.079	-39.079	0	0
2	M14	y	-39.079	-39.079	0	0
3	M15	y	-61.409	-61.409	0	0
4	M16	y	-61.409	-61.409	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	89.322	89.322	0	0
2	M14	y	68.481	68.481	0	0
3	M15	y	37.218	37.218	0	0
4	M16	y	37.218	37.218	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8												
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6												
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6										
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3										
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3										
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25										
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25										



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 20° 85mph 30psf 9.25ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	81.541	1	241.702	1	-.413	12	.014	1	-.005	15	.637	3
28			min	2.926	15	-228.159	3	-29.063	1	0	15	-.143	1	-.602	1
29		15	max	81.541	1	98.194	1	11.474	1	.014	1	-.005	12	.799	3
30			min	2.926	15	-87.322	3	.422	15	0	15	-.153	1	-.776	1
31		16	max	81.541	1	53.515	3	52.012	1	.014	1	-.004	12	.816	3
32			min	2.926	15	-45.314	1	1.866	15	0	15	-.12	1	-.803	1
33		17	max	81.541	1	194.352	3	92.549	1	.014	1	0	3	.689	3
34			min	2.926	15	-188.821	1	3.31	15	0	15	-.046	1	-.683	1
35		18	max	81.541	1	335.189	3	133.087	1	.014	1	.07	1	.417	3
36			min	2.926	15	-332.329	1	4.753	15	0	15	.003	15	-.415	1
37		19	max	81.541	1	476.026	3	173.624	1	.014	1	.228	1	0	1
38			min	2.926	15	-475.837	1	6.197	15	0	15	.008	15	0	3
39	M14	1	max	46.49	1	524.648	1	-6.433	15	.008	3	.269	1	0	1
40			min	1.672	15	-378.596	3	-180.223	1	-.014	1	.01	15	0	3
41		2	max	46.49	1	381.14	1	-4.989	15	.008	3	.104	1	.334	3
42			min	1.672	15	-272.031	3	-139.686	1	-.014	1	.004	15	-.465	1
43		3	max	46.49	1	237.633	1	-3.545	15	.008	3	.002	3	.559	3
44			min	1.672	15	-165.466	3	-99.148	1	-.014	1	-.019	1	-.783	1
45		4	max	46.49	1	94.125	1	-2.101	15	.008	3	-.003	12	.674	3
46			min	1.672	15	-58.9	3	-58.611	1	-.014	1	-.1	1	-.954	1
47		5	max	46.49	1	47.665	3	-.657	15	.008	3	-.005	12	.68	3
48			min	1.672	15	-49.383	1	-18.073	1	-.014	1	-.139	1	-.977	1
49		6	max	46.49	1	154.23	3	22.464	1	.008	3	-.005	15	.576	3
50			min	1.672	15	-192.89	1	.171	12	-.014	1	-.137	1	-.852	1
51		7	max	46.49	1	260.795	3	63.002	1	.008	3	-.003	15	.363	3
52			min	1.672	15	-336.398	1	1.639	12	-.014	1	-.093	1	-.58	1
53		8	max	46.49	1	367.36	3	103.54	1	.008	3	0	10	.04	3
54			min	1.672	15	-479.906	1	3.106	12	-.014	1	-.007	1	-.161	1
55		9	max	46.49	1	473.925	3	144.077	1	.008	3	.12	1	.406	1
56			min	1.672	15	-623.414	1	4.574	12	-.014	1	.002	12	-.392	3
57		10	max	46.49	1	580.49	3	184.615	1	.014	1	.289	1	1.121	1
58			min	1.672	15	-766.921	1	6.042	12	-.008	3	.007	12	-.934	3
59		11	max	46.49	1	623.414	1	-4.574	12	.014	1	.12	1	.406	1
60			min	1.672	15	-473.925	3	-144.077	1	-.008	3	.002	12	-.392	3
61		12	max	46.49	1	479.906	1	-3.106	12	.014	1	0	10	.04	3
62			min	1.672	15	-367.36	3	-103.54	1	-.008	3	-.007	1	-.161	1
63		13	max	46.49	1	336.398	1	-1.639	12	.014	1	-.003	15	.363	3
64			min	1.672	15	-260.795	3	-63.002	1	-.008	3	-.093	1	-.58	1
65		14	max	46.49	1	192.89	1	-.171	12	.014	1	-.005	15	.576	3
66			min	1.672	15	-154.23	3	-22.464	1	-.008	3	-.137	1	-.852	1
67		15	max	46.49	1	49.383	1	18.073	1	.014	1	-.005	12	.68	3
68			min	1.672	15	-47.665	3	.657	15	-.008	3	-.139	1	-.977	1
69		16	max	46.49	1	58.9	3	58.611	1	.014	1	-.003	12	.674	3
70			min	1.672	15	-94.125	1	2.101	15	-.008	3	-.1	1	-.954	1
71		17	max	46.49	1	165.466	3	99.148	1	.014	1	.002	3	.559	3
72			min	1.672	15	-237.633	1	3.545	15	-.008	3	-.019	1	-.783	1
73		18	max	46.49	1	272.031	3	139.686	1	.014	1	.104	1	.334	3
74			min	1.672	15	-381.14	1	4.989	15	-.008	3	.004	15	-.465	1
75		19	max	46.49	1	378.596	3	180.223	1	.014	1	.269	1	0	1
76			min	1.672	15	-524.648	1	6.433	15	-.008	3	.01	15	0	3
77	M15	1	max	-1.785	15	586.574	1	-6.43	15	.015	1	.268	1	0	1
78			min	-49.589	1	-205.499	3	-180.172	1	-.007	3	.01	15	0	3
79		2	max	-1.785	15	424.707	1	-4.986	15	.015	1	.104	1	.183	3
80			min	-49.589	1	-150.344	3	-139.634	1	-.007	3	.004	15	-.52	1
81		3	max	-1.785	15	262.839	1	-3.542	15	.015	1	.001	3	.309	3
82			min	-49.589	1	-95.189	3	-99.096	1	-.007	3	-.019	1	-.873	1
83		4	max	-1.785	15	100.971	1	-2.098	15	.015	1	-.003	12	.379	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-49.589	1	-40.034	3	-58.559	1	-.007	3	-.1	1	-1.06	1
85		5	max	-1.785	15	15.121	3	-.654	15	.015	1	-.005	12	.391	3
86			min	-49.589	1	-60.897	1	-18.021	1	-.007	3	-.139	1	-1.081	1
87		6	max	-1.785	15	70.276	3	22.516	1	.015	1	-.005	15	.347	3
88			min	-49.589	1	-222.765	1	.226	12	-.007	3	-.137	1	-.935	1
89		7	max	-1.785	15	125.43	3	63.054	1	.015	1	-.003	15	.247	3
90			min	-49.589	1	-384.633	1	1.694	12	-.007	3	-.093	1	-.623	1
91		8	max	-1.785	15	180.585	3	103.591	1	.015	1	0	10	.09	3
92			min	-49.589	1	-546.501	1	3.161	12	-.007	3	-.007	1	-.144	1
93		9	max	-1.785	15	235.74	3	144.129	1	.015	1	.12	1	.501	1
94			min	-49.589	1	-708.369	1	4.629	12	-.007	3	.002	12	-.124	3
95		10	max	-1.785	15	290.895	3	184.667	1	.007	3	.289	1	1.312	1
96			min	-49.589	1	-870.237	1	6.097	12	-.015	1	.008	12	-.395	3
97		11	max	-1.785	15	708.369	1	-4.629	12	.007	3	.12	1	.501	1
98			min	-49.589	1	-235.74	3	-144.129	1	-.015	1	.002	12	-.124	3
99		12	max	-1.785	15	546.501	1	-3.161	12	.007	3	0	10	.09	3
100			min	-49.589	1	-180.585	3	-103.591	1	-.015	1	-.007	1	-.144	1
101		13	max	-1.785	15	384.633	1	-1.694	12	.007	3	-.003	15	.247	3
102			min	-49.589	1	-125.43	3	-63.054	1	-.015	1	-.093	1	-.623	1
103		14	max	-1.785	15	222.765	1	-.226	12	.007	3	-.005	15	.347	3
104			min	-49.589	1	-70.276	3	-22.516	1	-.015	1	-.137	1	-.935	1
105		15	max	-1.785	15	60.897	1	18.021	1	.007	3	-.005	12	.391	3
106			min	-49.589	1	-15.121	3	.654	15	-.015	1	-.139	1	-1.081	1
107		16	max	-1.785	15	40.034	3	58.559	1	.007	3	-.003	12	.379	3
108			min	-49.589	1	-100.971	1	2.098	15	-.015	1	-.1	1	-1.06	1
109		17	max	-1.785	15	95.189	3	99.096	1	.007	3	.001	3	.309	3
110			min	-49.589	1	-262.839	1	3.542	15	-.015	1	-.019	1	-.873	1
111		18	max	-1.785	15	150.344	3	139.634	1	.007	3	.104	1	.183	3
112			min	-49.589	1	-424.707	1	4.986	15	-.015	1	.004	15	-.52	1
113		19	max	-1.785	15	205.499	3	180.172	1	.007	3	.268	1	0	1
114			min	-49.589	1	-586.574	1	6.43	15	-.015	1	.01	15	0	3
115	M16	1	max	-3.253	15	538.267	1	-6.21	15	.013	1	.23	1	0	1
116			min	-90.476	1	-181.019	3	-174.062	1	-.009	3	.008	15	0	3
117		2	max	-3.253	15	376.399	1	-4.766	15	.013	1	.072	1	.158	3
118			min	-90.476	1	-125.864	3	-133.525	1	-.009	3	.003	15	-.47	1
119		3	max	-3.253	15	214.531	1	-3.322	15	.013	1	0	12	.259	3
120			min	-90.476	1	-70.709	3	-92.987	1	-.009	3	-.044	1	-.774	1
121		4	max	-3.253	15	52.663	1	-1.878	15	.013	1	-.004	12	.303	3
122			min	-90.476	1	-15.554	3	-52.449	1	-.009	3	-.119	1	-.911	1
123		5	max	-3.253	15	39.6	3	-.435	15	.013	1	-.005	12	.291	3
124			min	-90.476	1	-109.204	1	-11.912	1	-.009	3	-.152	1	-.882	1
125		6	max	-3.253	15	94.755	3	28.626	1	.013	1	-.005	15	.222	3
126			min	-90.476	1	-271.072	1	.578	12	-.009	3	-.143	1	-.687	1
127		7	max	-3.253	15	149.91	3	69.163	1	.013	1	-.003	15	.096	3
128			min	-90.476	1	-432.94	1	2.045	12	-.009	3	-.093	1	-.325	1
129		8	max	-3.253	15	205.065	3	109.701	1	.013	1	0	2	.203	1
130			min	-90.476	1	-594.808	1	3.513	12	-.009	3	-.002	3	-.087	3
131		9	max	-3.253	15	260.22	3	150.238	1	.013	1	.132	1	.898	1
132			min	-90.476	1	-756.676	1	4.98	12	-.009	3	.003	12	-.326	3
133		10	max	-3.253	15	315.375	3	190.776	1	.009	3	.308	1	1.759	1
134			min	-90.476	1	-918.544	1	6.448	12	-.013	1	.009	12	-.621	3
135		11	max	-3.253	15	756.676	1	-4.98	12	.009	3	.132	1	.898	1
136			min	-90.476	1	-260.22	3	-150.238	1	-.013	1	.003	12	-.326	3
137		12	max	-3.253	15	594.808	1	-3.513	12	.009	3	0	2	.203	1
138			min	-90.476	1	-205.065	3	-109.701	1	-.013	1	-.002	3	-.087	3
139		13	max	-3.253	15	432.94	1	-2.045	12	.009	3	-.003	15	.096	3
140			min	-90.476	1	-149.91	3	-69.163	1	-.013	1	-.093	1	-.325	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-3.253	15	271.072	1	-.578	12	.009	3	-.005	15	.222	3
142		min	-90.476	1	-94.755	3	-28.626	1	-.013	1	-.143	1	-.687	1
143	15	max	-3.253	15	109.204	1	11.912	1	.009	3	-.005	12	.291	3
144		min	-90.476	1	-39.6	3	.435	15	-.013	1	-.152	1	-.882	1
145	16	max	-3.253	15	15.554	3	52.449	1	.009	3	-.004	12	.303	3
146		min	-90.476	1	-52.663	1	1.878	15	-.013	1	-.119	1	-.911	1
147	17	max	-3.253	15	70.709	3	92.987	1	.009	3	0	12	.259	3
148		min	-90.476	1	-214.531	1	3.322	15	-.013	1	-.044	1	-.774	1
149	18	max	-3.253	15	125.864	3	133.525	1	.009	3	.072	1	.158	3
150		min	-90.476	1	-376.399	1	4.766	15	-.013	1	.003	15	-.47	1
151	19	max	-3.253	15	181.019	3	174.062	1	.009	3	.23	1	0	1
152		min	-90.476	1	-538.267	1	6.21	15	-.013	1	.008	15	0	3
153	M2	1	max	1090.371	1	2.156	4	.963	1	0	3	0	3	1
154		min	-867.704	3	.507	15	.034	15	0	1	0	1	0	1
155	2	max	1090.787	1	2.147	4	.963	1	0	3	0	1	0	15
156		min	-867.392	3	.505	15	.034	15	0	1	0	15	0	4
157	3	max	1091.203	1	2.138	4	.963	1	0	3	0	1	0	15
158		min	-867.08	3	.503	15	.034	15	0	1	0	15	-.001	4
159	4	max	1091.619	1	2.13	4	.963	1	0	3	0	1	0	15
160		min	-866.768	3	.501	15	.034	15	0	1	0	15	-.002	4
161	5	max	1092.035	1	2.121	4	.963	1	0	3	.001	1	0	15
162		min	-866.456	3	.499	15	.034	15	0	1	0	15	-.002	4
163	6	max	1092.451	1	2.112	4	.963	1	0	3	.001	1	0	15
164		min	-866.145	3	.497	15	.034	15	0	1	0	15	-.003	4
165	7	max	1092.867	1	2.104	4	.963	1	0	3	.002	1	0	15
166		min	-865.833	3	.495	15	.034	15	0	1	0	15	-.004	4
167	8	max	1093.283	1	2.095	4	.963	1	0	3	.002	1	0	15
168		min	-865.521	3	.493	15	.034	15	0	1	0	15	-.004	4
169	9	max	1093.698	1	2.086	4	.963	1	0	3	.002	1	-.001	15
170		min	-865.209	3	.491	15	.034	15	0	1	0	15	-.005	4
171	10	max	1094.114	1	2.077	4	.963	1	0	3	.002	1	-.001	15
172		min	-864.897	3	.488	15	.034	15	0	1	0	15	-.005	4
173	11	max	1094.53	1	2.069	4	.963	1	0	3	.003	1	-.001	15
174		min	-864.585	3	.486	15	.034	15	0	1	0	15	-.006	4
175	12	max	1094.946	1	2.06	4	.963	1	0	3	.003	1	-.002	15
176		min	-864.273	3	.484	15	.034	15	0	1	0	15	-.007	4
177	13	max	1095.362	1	2.051	4	.963	1	0	3	.003	1	-.002	15
178		min	-863.961	3	.482	15	.034	15	0	1	0	15	-.007	4
179	14	max	1095.778	1	2.043	4	.963	1	0	3	.003	1	-.002	15
180		min	-863.649	3	.48	15	.034	15	0	1	0	15	-.008	4
181	15	max	1096.194	1	2.034	4	.963	1	0	3	.004	1	-.002	15
182		min	-863.337	3	.478	15	.034	15	0	1	0	15	-.008	4
183	16	max	1096.61	1	2.025	4	.963	1	0	3	.004	1	-.002	15
184		min	-863.025	3	.476	15	.034	15	0	1	0	15	-.009	4
185	17	max	1097.026	1	2.016	4	.963	1	0	3	.004	1	-.002	15
186		min	-862.714	3	.474	15	.034	15	0	1	0	15	-.009	4
187	18	max	1097.441	1	2.008	4	.963	1	0	3	.005	1	-.002	15
188		min	-862.402	3	.472	15	.034	15	0	1	0	15	-.01	4
189	19	max	1097.857	1	1.999	4	.963	1	0	3	.005	1	-.002	15
190		min	-862.09	3	.47	15	.034	15	0	1	0	15	-.01	4
191	M3	1	max	338.468	2	9.1	4	.221	1	0	5	0	1	4
192		min	-463.397	3	2.139	15	.008	15	0	1	0	15	.002	15
193	2	max	338.297	2	8.226	4	.221	1	0	5	0	1	.006	4
194		min	-463.525	3	1.934	15	.008	15	0	1	0	15	.002	15
195	3	max	338.127	2	7.351	4	.221	1	0	5	0	1	.003	2
196		min	-463.652	3	1.728	15	.008	15	0	1	0	15	0	12
197	4	max	337.957	2	6.477	4	.221	1	0	5	0	1	0	2





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-463.78	3	1.523	15	.008	15	0	1	0	15	-.001	3
199		5	max	337.786	2	5.602	4	.221	1	0	5	0	1	0	15
200			min	-463.908	3	1.317	15	.008	15	0	1	0	15	-.003	4
201		6	max	337.616	2	4.728	4	.221	1	0	5	0	1	-.001	15
202			min	-464.036	3	1.111	15	.008	15	0	1	0	15	-.006	4
203		7	max	337.446	2	3.854	4	.221	1	0	5	0	1	-.002	15
204			min	-464.163	3	.906	15	.008	15	0	1	0	15	-.008	4
205		8	max	337.275	2	2.979	4	.221	1	0	5	0	1	-.002	15
206			min	-464.291	3	.7	15	.008	15	0	1	0	15	-.01	4
207		9	max	337.105	2	2.105	4	.221	1	0	5	0	1	-.003	15
208			min	-464.419	3	.495	15	.008	15	0	1	0	15	-.011	4
209		10	max	336.935	2	1.23	4	.221	1	0	5	.001	1	-.003	15
210			min	-464.547	3	.289	15	.008	15	0	1	0	15	-.012	4
211		11	max	336.764	2	.386	2	.221	1	0	5	.001	1	-.003	15
212			min	-464.674	3	.044	12	.008	15	0	1	0	15	-.012	4
213		12	max	336.594	2	-.122	15	.221	1	0	5	.001	1	-.003	15
214			min	-464.802	3	-.519	4	.008	15	0	1	0	15	-.012	4
215		13	max	336.424	2	-.327	15	.221	1	0	5	.001	1	-.003	15
216			min	-464.93	3	-1.393	4	.008	15	0	1	0	15	-.011	4
217		14	max	336.253	2	-.533	15	.221	1	0	5	.001	1	-.002	15
218			min	-465.058	3	-2.268	4	.008	15	0	1	0	15	-.011	4
219		15	max	336.083	2	-.738	15	.221	1	0	5	.002	1	-.002	15
220			min	-465.185	3	-3.142	4	.008	15	0	1	0	15	-.009	4
221		16	max	335.912	2	-.944	15	.221	1	0	5	.002	1	-.002	15
222			min	-465.313	3	-4.016	4	.008	15	0	1	0	15	-.008	4
223		17	max	335.742	2	-1.15	15	.221	1	0	5	.002	1	-.001	15
224			min	-465.441	3	-4.891	4	.008	15	0	1	0	15	-.005	4
225		18	max	335.572	2	-1.355	15	.221	1	0	5	.002	1	0	15
226			min	-465.569	3	-5.765	4	.008	15	0	1	0	15	-.003	4
227		19	max	335.401	2	-1.561	15	.221	1	0	5	.002	1	0	1
228			min	-465.696	3	-6.64	4	.008	15	0	1	0	15	0	1
229	M4	1	max	1174.265	1	0	1	-.482	15	0	1	.001	1	0	1
230			min	-144.659	3	0	1	-13.5	1	0	1	0	15	0	1
231		2	max	1174.436	1	0	1	-.482	15	0	1	0	12	0	1
232			min	-144.531	3	0	1	-13.5	1	0	1	0	1	0	1
233		3	max	1174.606	1	0	1	-.482	15	0	1	0	15	0	1
234			min	-144.403	3	0	1	-13.5	1	0	1	-.002	1	0	1
235		4	max	1174.776	1	0	1	-.482	15	0	1	0	15	0	1
236			min	-144.276	3	0	1	-13.5	1	0	1	-.003	1	0	1
237		5	max	1174.947	1	0	1	-.482	15	0	1	0	15	0	1
238			min	-144.148	3	0	1	-13.5	1	0	1	-.005	1	0	1
239		6	max	1175.117	1	0	1	-.482	15	0	1	0	15	0	1
240			min	-144.02	3	0	1	-13.5	1	0	1	-.007	1	0	1
241		7	max	1175.287	1	0	1	-.482	15	0	1	0	15	0	1
242			min	-143.892	3	0	1	-13.5	1	0	1	-.008	1	0	1
243		8	max	1175.458	1	0	1	-.482	15	0	1	0	15	0	1
244			min	-143.765	3	0	1	-13.5	1	0	1	-.01	1	0	1
245		9	max	1175.628	1	0	1	-.482	15	0	1	0	15	0	1
246			min	-143.637	3	0	1	-13.5	1	0	1	-.011	1	0	1
247		10	max	1175.798	1	0	1	-.482	15	0	1	0	15	0	1
248			min	-143.509	3	0	1	-13.5	1	0	1	-.013	1	0	1
249		11	max	1175.969	1	0	1	-.482	15	0	1	0	15	0	1
250			min	-143.381	3	0	1	-13.5	1	0	1	-.014	1	0	1
251		12	max	1176.139	1	0	1	-.482	15	0	1	0	15	0	1
252			min	-143.254	3	0	1	-13.5	1	0	1	-.016	1	0	1
253		13	max	1176.309	1	0	1	-.482	15	0	1	0	15	0	1
254			min	-143.126	3	0	1	-13.5	1	0	1	-.017	1	0	1





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Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1176.48	1	0	1	-.482	15	0	1	0	15	0	1
256		min	-142.998	3	0	1	-13.5	1	0	1	-.019	1	0	1
257	15	max	1176.65	1	0	1	-.482	15	0	1	0	15	0	1
258		min	-142.87	3	0	1	-13.5	1	0	1	-.021	1	0	1
259	16	max	1176.82	1	0	1	-.482	15	0	1	0	15	0	1
260		min	-142.743	3	0	1	-13.5	1	0	1	-.022	1	0	1
261	17	max	1176.991	1	0	1	-.482	15	0	1	0	15	0	1
262		min	-142.615	3	0	1	-13.5	1	0	1	-.024	1	0	1
263	18	max	1177.161	1	0	1	-.482	15	0	1	0	15	0	1
264		min	-142.487	3	0	1	-13.5	1	0	1	-.025	1	0	1
265	19	max	1177.332	1	0	1	-.482	15	0	1	0	15	0	1
266		min	-142.359	3	0	1	-13.5	1	0	1	-.027	1	0	1
267	M6	1	max	3424.958	1	2.367	2	0	1	0	0	1	0	1
268		min	-2795.711	3	.344	12	0	1	0	1	0	1	0	1
269	2	max	3425.374	1	2.36	2	0	1	0	1	0	1	0	12
270		min	-2795.399	3	.341	12	0	1	0	1	0	1	0	2
271	3	max	3425.79	1	2.354	2	0	1	0	1	0	1	0	12
272		min	-2795.087	3	.337	12	0	1	0	1	0	1	-.001	2
273	4	max	3426.206	1	2.347	2	0	1	0	1	0	1	0	12
274		min	-2794.775	3	.334	12	0	1	0	1	0	1	-.002	2
275	5	max	3426.622	1	2.34	2	0	1	0	1	0	1	0	12
276		min	-2794.463	3	.33	12	0	1	0	1	0	1	-.003	2
277	6	max	3427.038	1	2.333	2	0	1	0	1	0	1	0	12
278		min	-2794.151	3	.327	12	0	1	0	1	0	1	-.003	2
279	7	max	3427.454	1	2.326	2	0	1	0	1	0	1	0	12
280		min	-2793.839	3	.324	12	0	1	0	1	0	1	-.004	2
281	8	max	3427.87	1	2.32	2	0	1	0	1	0	1	0	12
282		min	-2793.527	3	.32	12	0	1	0	1	0	1	-.005	2
283	9	max	3428.285	1	2.313	2	0	1	0	1	0	1	0	12
284		min	-2793.215	3	.317	12	0	1	0	1	0	1	-.005	2
285	10	max	3428.701	1	2.306	2	0	1	0	1	0	1	0	12
286		min	-2792.903	3	.313	12	0	1	0	1	0	1	-.006	2
287	11	max	3429.117	1	2.299	2	0	1	0	1	0	1	0	12
288		min	-2792.591	3	.31	12	0	1	0	1	0	1	-.007	2
289	12	max	3429.533	1	2.292	2	0	1	0	1	0	1	-.001	12
290		min	-2792.28	3	.307	12	0	1	0	1	0	1	-.007	2
291	13	max	3429.949	1	2.286	2	0	1	0	1	0	1	-.001	12
292		min	-2791.968	3	.303	12	0	1	0	1	0	1	-.008	2
293	14	max	3430.365	1	2.279	2	0	1	0	1	0	1	-.001	12
294		min	-2791.656	3	.3	12	0	1	0	1	0	1	-.008	2
295	15	max	3430.781	1	2.272	2	0	1	0	1	0	1	-.001	12
296		min	-2791.344	3	.296	12	0	1	0	1	0	1	-.009	2
297	16	max	3431.197	1	2.265	2	0	1	0	1	0	1	-.001	12
298		min	-2791.032	3	.293	12	0	1	0	1	0	1	-.01	2
299	17	max	3431.613	1	2.259	2	0	1	0	1	0	1	-.001	12
300		min	-2790.72	3	.29	12	0	1	0	1	0	1	-.01	2
301	18	max	3432.028	1	2.252	2	0	1	0	1	0	1	-.002	12
302		min	-2790.408	3	.286	12	0	1	0	1	0	1	-.011	2
303	19	max	3432.444	1	2.245	2	0	1	0	1	0	1	-.002	12
304		min	-2790.096	3	.283	12	0	1	0	1	0	1	-.012	2
305	M7	1	max	1348.98	2	9.143	4	0	1	0	0	1	.012	2
306		min	-1417.42	3	2.145	15	0	1	0	1	0	1	.002	12
307	2	max	1348.81	2	8.268	4	0	1	0	1	0	1	.008	2
308		min	-1417.548	3	1.939	15	0	1	0	1	0	1	0	3
309	3	max	1348.64	2	7.394	4	0	1	0	1	0	1	.005	2
310		min	-1417.676	3	1.734	15	0	1	0	1	0	1	-.002	3
311	4	max	1348.469	2	6.519	4	0	1	0	1	0	1	.002	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-1417.803	3	1.528	15	0	1	0	1	0	1	-.004	3
313	5	max	1348.299	2	5.645	4	0	1	0	1	0	1	0	2
314		min	-1417.931	3	1.323	15	0	1	0	1	0	1	-.005	3
315	6	max	1348.129	2	4.77	4	0	1	0	1	0	1	-.001	15
316		min	-1418.059	3	1.117	15	0	1	0	1	0	1	-.006	3
317	7	max	1347.958	2	3.896	4	0	1	0	1	0	1	-.002	15
318		min	-1418.187	3	.912	15	0	1	0	1	0	1	-.008	4
319	8	max	1347.788	2	3.022	4	0	1	0	1	0	1	-.002	15
320		min	-1418.314	3	.706	15	0	1	0	1	0	1	-.009	4
321	9	max	1347.617	2	2.147	4	0	1	0	1	0	1	-.002	15
322		min	-1418.442	3	.501	15	0	1	0	1	0	1	-.011	4
323	10	max	1347.447	2	1.366	2	0	1	0	1	0	1	-.003	15
324		min	-1418.57	3	.186	12	0	1	0	1	0	1	-.011	4
325	11	max	1347.277	2	.684	2	0	1	0	1	0	1	-.003	15
326		min	-1418.698	3	-.262	3	0	1	0	1	0	1	-.012	4
327	12	max	1347.106	2	.003	2	0	1	0	1	0	1	-.003	15
328		min	-1418.826	3	-.773	3	0	1	0	1	0	1	-.012	4
329	13	max	1346.936	2	-.322	15	0	1	0	1	0	1	-.003	15
330		min	-1418.953	3	-1.351	4	0	1	0	1	0	1	-.011	4
331	14	max	1346.766	2	-.527	15	0	1	0	1	0	1	-.002	15
332		min	-1419.081	3	-2.225	4	0	1	0	1	0	1	-.01	4
333	15	max	1346.595	2	-.733	15	0	1	0	1	0	1	-.002	15
334		min	-1419.209	3	-3.1	4	0	1	0	1	0	1	-.009	4
335	16	max	1346.425	2	-.938	15	0	1	0	1	0	1	-.002	15
336		min	-1419.337	3	-3.974	4	0	1	0	1	0	1	-.008	4
337	17	max	1346.255	2	-1.144	15	0	1	0	1	0	1	-.001	15
338		min	-1419.464	3	-4.848	4	0	1	0	1	0	1	-.005	4
339	18	max	1346.084	2	-1.349	15	0	1	0	1	0	1	0	15
340		min	-1419.592	3	-5.723	4	0	1	0	1	0	1	-.003	4
341	19	max	1345.914	2	-1.555	15	0	1	0	1	0	1	0	1
342		min	-1419.72	3	-6.597	4	0	1	0	1	0	1	0	1
343	M8	1	max	3251.551	1	0	1	0	1	0	1	0	1	1
344		min	-525.973	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3251.722	1	0	1	0	1	0	1	0	1	0	1
346		min	-525.845	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3251.892	1	0	1	0	1	0	1	0	1	0	1
348		min	-525.717	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3252.062	1	0	1	0	1	0	1	0	1	0	1
350		min	-525.589	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3252.233	1	0	1	0	1	0	1	0	1	0	1
352		min	-525.462	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3252.403	1	0	1	0	1	0	1	0	1	0	1
354		min	-525.334	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3252.573	1	0	1	0	1	0	1	0	1	0	1
356		min	-525.206	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3252.744	1	0	1	0	1	0	1	0	1	0	1
358		min	-525.078	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3252.914	1	0	1	0	1	0	1	0	1	0	1
360		min	-524.951	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3253.084	1	0	1	0	1	0	1	0	1	0	1
362		min	-524.823	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3253.255	1	0	1	0	1	0	1	0	1	0	1
364		min	-524.695	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3253.425	1	0	1	0	1	0	1	0	1	0	1
366		min	-524.567	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3253.595	1	0	1	0	1	0	1	0	1	0	1
368		min	-524.44	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3253.766	1	0	1	0	1	0	1	0	1	0	1
370			min	-524.312	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3253.936	1	0	1	0	1	0	1	0	1	0	1
372			min	-524.184	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3254.106	1	0	1	0	1	0	1	0	1	0	1
374			min	-524.056	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3254.277	1	0	1	0	1	0	1	0	1	0	1
376			min	-523.929	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3254.447	1	0	1	0	1	0	1	0	1	0	1
378			min	-523.801	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3254.617	1	0	1	0	1	0	1	0	1	0	1
380			min	-523.673	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1090.371	1	2.156	4	-.034	15	0	1	0	1	0	1
382			min	-867.704	3	.507	15	-.963	1	0	3	0	3	0	1
383		2	max	1090.787	1	2.147	4	-.034	15	0	1	0	15	0	15
384			min	-867.392	3	.505	15	-.963	1	0	3	0	1	0	4
385		3	max	1091.203	1	2.138	4	-.034	15	0	1	0	15	0	15
386			min	-867.08	3	.503	15	-.963	1	0	3	0	1	-.001	4
387		4	max	1091.619	1	2.13	4	-.034	15	0	1	0	15	0	15
388			min	-866.768	3	.501	15	-.963	1	0	3	0	1	-.002	4
389		5	max	1092.035	1	2.121	4	-.034	15	0	1	0	15	0	15
390			min	-866.456	3	.499	15	-.963	1	0	3	-.001	1	-.002	4
391		6	max	1092.451	1	2.112	4	-.034	15	0	1	0	15	0	15
392			min	-866.145	3	.497	15	-.963	1	0	3	-.001	1	-.003	4
393		7	max	1092.867	1	2.104	4	-.034	15	0	1	0	15	0	15
394			min	-865.833	3	.495	15	-.963	1	0	3	-.002	1	-.004	4
395		8	max	1093.283	1	2.095	4	-.034	15	0	1	0	15	0	15
396			min	-865.521	3	.493	15	-.963	1	0	3	-.002	1	-.004	4
397		9	max	1093.698	1	2.086	4	-.034	15	0	1	0	15	-.001	15
398			min	-865.209	3	.491	15	-.963	1	0	3	-.002	1	-.005	4
399		10	max	1094.114	1	2.077	4	-.034	15	0	1	0	15	-.001	15
400			min	-864.897	3	.488	15	-.963	1	0	3	-.002	1	-.005	4
401		11	max	1094.53	1	2.069	4	-.034	15	0	1	0	15	-.001	15
402			min	-864.585	3	.486	15	-.963	1	0	3	-.003	1	-.006	4
403		12	max	1094.946	1	2.06	4	-.034	15	0	1	0	15	-.002	15
404			min	-864.273	3	.484	15	-.963	1	0	3	-.003	1	-.007	4
405		13	max	1095.362	1	2.051	4	-.034	15	0	1	0	15	-.002	15
406			min	-863.961	3	.482	15	-.963	1	0	3	-.003	1	-.007	4
407		14	max	1095.778	1	2.043	4	-.034	15	0	1	0	15	-.002	15
408			min	-863.649	3	.48	15	-.963	1	0	3	-.003	1	-.008	4
409		15	max	1096.194	1	2.034	4	-.034	15	0	1	0	15	-.002	15
410			min	-863.337	3	.478	15	-.963	1	0	3	-.004	1	-.008	4
411		16	max	1096.61	1	2.025	4	-.034	15	0	1	0	15	-.002	15
412			min	-863.025	3	.476	15	-.963	1	0	3	-.004	1	-.009	4
413		17	max	1097.026	1	2.016	4	-.034	15	0	1	0	15	-.002	15
414			min	-862.714	3	.474	15	-.963	1	0	3	-.004	1	-.009	4
415		18	max	1097.441	1	2.008	4	-.034	15	0	1	0	15	-.002	15
416			min	-862.402	3	.472	15	-.963	1	0	3	-.005	1	-.01	4
417		19	max	1097.857	1	1.999	4	-.034	15	0	1	0	15	-.002	15
418			min	-862.09	3	.47	15	-.963	1	0	3	-.005	1	-.01	4
419	M11	1	max	338.468	2	9.1	4	-.008	15	0	1	0	15	.01	4
420			min	-463.397	3	2.139	15	-.221	1	0	5	0	1	.002	15
421		2	max	338.297	2	8.226	4	-.008	15	0	1	0	15	.006	4
422			min	-463.525	3	1.934	15	-.221	1	0	5	0	1	.002	15
423		3	max	338.127	2	7.351	4	-.008	15	0	1	0	15	.003	2
424			min	-463.652	3	1.728	15	-.221	1	0	5	0	1	0	12
425		4	max	337.957	2	6.477	4	-.008	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-463.78	3	1.523	15	- .221	1	0	5	0	1	- .001	3
427		5	max	337.786	2	5.602	4	- .008	15	0	1	0	15	0	15
428			min	-463.908	3	1.317	15	- .221	1	0	5	0	1	- .003	4
429		6	max	337.616	2	4.728	4	- .008	15	0	1	0	15	- .001	15
430			min	-464.036	3	1.111	15	- .221	1	0	5	0	1	- .006	4
431		7	max	337.446	2	3.854	4	- .008	15	0	1	0	15	- .002	15
432			min	-464.163	3	.906	15	- .221	1	0	5	0	1	- .008	4
433		8	max	337.275	2	2.979	4	- .008	15	0	1	0	15	- .002	15
434			min	-464.291	3	.7	15	- .221	1	0	5	0	1	- .01	4
435		9	max	337.105	2	2.105	4	- .008	15	0	1	0	15	- .003	15
436			min	-464.419	3	.495	15	- .221	1	0	5	0	1	- .011	4
437		10	max	336.935	2	1.23	4	- .008	15	0	1	0	15	- .003	15
438			min	-464.547	3	.289	15	- .221	1	0	5	- .001	1	- .012	4
439		11	max	336.764	2	.386	2	- .008	15	0	1	0	15	- .003	15
440			min	-464.674	3	.044	12	- .221	1	0	5	- .001	1	- .012	4
441		12	max	336.594	2	- .122	15	- .008	15	0	1	0	15	- .003	15
442			min	-464.802	3	- .519	4	- .221	1	0	5	- .001	1	- .012	4
443		13	max	336.424	2	- .327	15	- .008	15	0	1	0	15	- .003	15
444			min	-464.93	3	-1.393	4	- .221	1	0	5	- .001	1	- .011	4
445		14	max	336.253	2	- .533	15	- .008	15	0	1	0	15	- .002	15
446			min	-465.058	3	-2.268	4	- .221	1	0	5	- .001	1	- .011	4
447		15	max	336.083	2	- .738	15	- .008	15	0	1	0	15	- .002	15
448			min	-465.185	3	-3.142	4	- .221	1	0	5	- .002	1	- .009	4
449		16	max	335.912	2	- .944	15	- .008	15	0	1	0	15	- .002	15
450			min	-465.313	3	-4.016	4	- .221	1	0	5	- .002	1	- .008	4
451		17	max	335.742	2	-1.15	15	- .008	15	0	1	0	15	- .001	15
452			min	-465.441	3	-4.891	4	- .221	1	0	5	- .002	1	- .005	4
453		18	max	335.572	2	-1.355	15	- .008	15	0	1	0	15	0	15
454			min	-465.569	3	-5.765	4	- .221	1	0	5	- .002	1	- .003	4
455		19	max	335.401	2	-1.561	15	- .008	15	0	1	0	15	0	1
456			min	-465.696	3	-6.64	4	- .221	1	0	5	- .002	1	0	1
457	M12	1	max	1174.265	1	0	1	13.5	1	0	1	0	15	0	1
458			min	-144.659	3	0	1	.482	15	0	1	- .001	1	0	1
459		2	max	1174.436	1	0	1	13.5	1	0	1	0	1	0	1
460			min	-144.531	3	0	1	.482	15	0	1	0	12	0	1
461		3	max	1174.606	1	0	1	13.5	1	0	1	.002	1	0	1
462			min	-144.403	3	0	1	.482	15	0	1	0	15	0	1
463		4	max	1174.776	1	0	1	13.5	1	0	1	.003	1	0	1
464			min	-144.276	3	0	1	.482	15	0	1	0	15	0	1
465		5	max	1174.947	1	0	1	13.5	1	0	1	.005	1	0	1
466			min	-144.148	3	0	1	.482	15	0	1	0	15	0	1
467		6	max	1175.117	1	0	1	13.5	1	0	1	.007	1	0	1
468			min	-144.02	3	0	1	.482	15	0	1	0	15	0	1
469		7	max	1175.287	1	0	1	13.5	1	0	1	.008	1	0	1
470			min	-143.892	3	0	1	.482	15	0	1	0	15	0	1
471		8	max	1175.458	1	0	1	13.5	1	0	1	.01	1	0	1
472			min	-143.765	3	0	1	.482	15	0	1	0	15	0	1
473		9	max	1175.628	1	0	1	13.5	1	0	1	.011	1	0	1
474			min	-143.637	3	0	1	.482	15	0	1	0	15	0	1
475		10	max	1175.798	1	0	1	13.5	1	0	1	.013	1	0	1
476			min	-143.509	3	0	1	.482	15	0	1	0	15	0	1
477		11	max	1175.969	1	0	1	13.5	1	0	1	.014	1	0	1
478			min	-143.381	3	0	1	.482	15	0	1	0	15	0	1
479		12	max	1176.139	1	0	1	13.5	1	0	1	.016	1	0	1
480			min	-143.254	3	0	1	.482	15	0	1	0	15	0	1
481		13	max	1176.309	1	0	1	13.5	1	0	1	.017	1	0	1
482			min	-143.126	3	0	1	.482	15	0	1	0	15	0	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1176.48	1	0	1	13.5	1	0	1	.019	1	0	1
484			min	-142.998	3	0	1	.482	15	0	1	0	15	0	1
485		15	max	1176.65	1	0	1	13.5	1	0	1	.021	1	0	1
486			min	-142.87	3	0	1	.482	15	0	1	0	15	0	1
487		16	max	1176.82	1	0	1	13.5	1	0	1	.022	1	0	1
488			min	-142.743	3	0	1	.482	15	0	1	0	15	0	1
489		17	max	1176.991	1	0	1	13.5	1	0	1	.024	1	0	1
490			min	-142.615	3	0	1	.482	15	0	1	0	15	0	1
491		18	max	1177.161	1	0	1	13.5	1	0	1	.025	1	0	1
492			min	-142.487	3	0	1	.482	15	0	1	0	15	0	1
493		19	max	1177.332	1	0	1	13.5	1	0	1	.027	1	0	1
494			min	-142.359	3	0	1	.482	15	0	1	0	15	0	1
495	M1	1	max	173.629	1	476	3	-2.926	15	0	1	.228	1	0	15
496			min	6.197	15	-473.674	1	-81.407	1	0	3	.008	15	-.014	1
497		2	max	174.206	1	474.813	3	-2.926	15	0	1	.177	1	.28	1
498			min	6.371	15	-475.257	1	-81.407	1	0	3	.006	15	-.296	3
499		3	max	295.574	3	549.459	1	-2.892	15	0	3	.127	1	.564	1
500			min	-205.614	2	-347.893	3	-80.721	1	0	1	.005	15	-.582	3
501		4	max	296.006	3	547.876	1	-2.892	15	0	3	.077	1	.224	1
502			min	-205.038	2	-349.08	3	-80.721	1	0	1	.003	15	-.366	3
503		5	max	296.438	3	546.293	1	-2.892	15	0	3	.027	1	-.005	15
504			min	-204.462	2	-350.268	3	-80.721	1	0	1	0	15	-.148	3
505		6	max	296.871	3	544.71	1	-2.892	15	0	3	0	15	.069	3
506			min	-203.886	2	-351.455	3	-80.721	1	0	1	-.023	1	-.454	1
507		7	max	297.303	3	543.126	1	-2.892	15	0	3	-.003	15	.288	3
508			min	-203.309	2	-352.642	3	-80.721	1	0	1	-.073	1	-.792	1
509		8	max	297.735	3	541.543	1	-2.892	15	0	3	-.004	15	.507	3
510			min	-202.733	2	-353.83	3	-80.721	1	0	1	-.124	1	-1.128	1
511		9	max	308.978	3	31.928	2	-4.564	15	0	9	.078	1	.594	3
512			min	-131.793	2	.482	15	-127.259	1	0	3	.003	15	-1.284	1
513		10	max	309.411	3	30.345	2	-4.564	15	0	9	0	15	.578	3
514			min	-131.217	2	.004	15	-127.259	1	0	3	-.001	1	-1.295	1
515		11	max	309.843	3	28.762	2	-4.564	15	0	9	-.003	15	.564	3
516			min	-130.641	2	-1.918	4	-127.259	1	0	3	-.08	1	-1.305	1
517		12	max	320.999	3	229.202	3	-2.779	15	0	1	.121	1	.492	3
518			min	-76.017	10	-576.178	1	-77.718	1	0	3	.004	15	-1.153	1
519		13	max	321.431	3	228.015	3	-2.779	15	0	1	.073	1	.35	3
520			min	-75.537	10	-577.761	1	-77.718	1	0	3	.003	15	-.794	1
521		14	max	321.864	3	226.828	3	-2.779	15	0	1	.025	1	.209	3
522			min	-75.057	10	-579.344	1	-77.718	1	0	3	0	15	-.435	1
523		15	max	322.296	3	225.64	3	-2.779	15	0	1	0	15	.068	3
524			min	-74.577	10	-580.927	1	-77.718	1	0	3	-.024	1	-.075	1
525		16	max	322.728	3	224.453	3	-2.779	15	0	1	-.003	15	.286	1
526			min	-74.097	10	-582.51	1	-77.718	1	0	3	-.072	1	-.071	3
527		17	max	323.16	3	223.265	3	-2.779	15	0	1	-.004	15	.648	1
528			min	-73.617	10	-584.094	1	-77.718	1	0	3	-.12	1	-.21	3
529		18	max	-6.384	15	541.907	1	-3.253	15	0	3	-.006	15	.323	1
530			min	-174.634	1	-179.888	3	-90.602	1	0	1	-.174	1	-.103	3
531		19	max	-6.21	15	540.324	1	-3.253	15	0	3	-.008	15	.009	3
532			min	-174.058	1	-181.075	3	-90.602	1	0	1	-.23	1	-.013	1
533	M5	1	max	382.417	1	1582.963	3	0	1	0	1	0	1	.029	1
534			min	12.566	12	-1620.992	1	0	1	0	1	0	1	0	15
535		2	max	382.994	1	1581.776	3	0	1	0	1	0	1	1.035	1
536			min	12.854	12	-1622.575	1	0	1	0	1	0	1	-.979	3
537		3	max	932.121	3	1577.832	1	0	1	0	1	0	1	2.008	1
538			min	-733.707	1	-1076.111	3	0	1	0	1	0	1	-1.931	3
539		4	max	932.553	3	1576.248	1	0	1	0	1	0	1	1.029	1





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-733.131	1	-1077.298	3	0	1	0	1	0	1	-1.263	3
541		5	max	932.986	3	1574.665	1	0	1	0	1	0	1	.051	1
542			min	-732.555	1	-1078.485	3	0	1	0	1	0	1	-.594	3
543		6	max	933.418	3	1573.082	1	0	1	0	1	0	1	.075	3
544			min	-731.979	1	-1079.673	3	0	1	0	1	0	1	-.925	1
545		7	max	933.85	3	1571.499	1	0	1	0	1	0	1	.746	3
546			min	-731.403	1	-1080.86	3	0	1	0	1	0	1	-1.901	1
547		8	max	934.282	3	1569.916	1	0	1	0	1	0	1	1.417	3
548			min	-730.826	1	-1082.047	3	0	1	0	1	0	1	-2.876	1
549		9	max	952.716	3	105.988	2	0	1	0	1	0	1	1.636	3
550			min	-559.434	2	.48	15	0	1	0	1	0	1	-3.26	1
551		10	max	953.148	3	104.405	2	0	1	0	1	0	1	1.58	3
552			min	-558.858	2	.002	15	0	1	0	1	0	1	-3.296	1
553		11	max	953.58	3	102.822	2	0	1	0	1	0	1	1.524	3
554			min	-558.282	2	-1.707	4	0	1	0	1	0	1	-3.332	1
555		12	max	972.187	3	685.706	3	0	1	0	1	0	1	1.334	3
556			min	-411.203	2	-1685.682	1	0	1	0	1	0	1	-2.962	1
557		13	max	972.62	3	684.518	3	0	1	0	1	0	1	.909	3
558			min	-410.627	2	-1687.265	1	0	1	0	1	0	1	-1.916	1
559		14	max	973.052	3	683.331	3	0	1	0	1	0	1	.484	3
560			min	-410.05	2	-1688.848	1	0	1	0	1	0	1	-.868	1
561		15	max	973.484	3	682.143	3	0	1	0	1	0	1	.22	2
562			min	-409.474	2	-1690.432	1	0	1	0	1	0	1	0	5
563		16	max	973.916	3	680.956	3	0	1	0	1	0	1	1.23	1
564			min	-408.898	2	-1692.015	1	0	1	0	1	0	1	-.362	3
565		17	max	974.348	3	679.769	3	0	1	0	1	0	1	2.281	1
566			min	-408.322	2	-1693.598	1	0	1	0	1	0	1	-.785	3
567		18	max	-13.184	12	1848.414	1	0	1	0	1	0	1	1.172	1
568			min	-382.136	1	-629.841	3	0	1	0	1	0	1	-.408	3
569		19	max	-12.896	12	1846.83	1	0	1	0	1	0	1	.025	1
570			min	-381.56	1	-631.029	3	0	1	0	1	0	1	-.017	3
571	M9	1	max	173.629	1	476	3	81.407	1	0	3	-.008	15	0	15
572			min	6.197	15	-473.674	1	2.926	15	0	1	-.228	1	-.014	1
573		2	max	174.206	1	474.813	3	81.407	1	0	3	-.006	15	.28	1
574			min	6.371	15	-475.257	1	2.926	15	0	1	-.177	1	-.296	3
575		3	max	295.574	3	549.459	1	80.721	1	0	1	-.005	15	.564	1
576			min	-205.614	2	-347.893	3	2.892	15	0	3	-.127	1	-.582	3
577		4	max	296.006	3	547.876	1	80.721	1	0	1	-.003	15	.224	1
578			min	-205.038	2	-349.08	3	2.892	15	0	3	-.077	1	-.366	3
579		5	max	296.438	3	546.293	1	80.721	1	0	1	0	15	-.005	15
580			min	-204.462	2	-350.268	3	2.892	15	0	3	-.027	1	-.148	3
581		6	max	296.871	3	544.71	1	80.721	1	0	1	.023	1	.069	3
582			min	-203.886	2	-351.455	3	2.892	15	0	3	0	15	-.454	1
583		7	max	297.303	3	543.126	1	80.721	1	0	1	.073	1	.288	3
584			min	-203.309	2	-352.642	3	2.892	15	0	3	.003	15	-.792	1
585		8	max	297.735	3	541.543	1	80.721	1	0	1	.124	1	.507	3
586			min	-202.733	2	-353.83	3	2.892	15	0	3	.004	15	-1.128	1
587		9	max	308.978	3	31.928	2	127.259	1	0	3	-.003	15	.594	3
588			min	-131.793	2	.482	15	4.564	15	0	9	-.078	1	-1.284	1
589		10	max	309.411	3	30.345	2	127.259	1	0	3	.001	1	.578	3
590			min	-131.217	2	.004	15	4.564	15	0	9	0	15	-1.295	1
591		11	max	309.843	3	28.762	2	127.259	1	0	3	.08	1	.564	3
592			min	-130.641	2	-1.918	4	4.564	15	0	9	.003	15	-1.305	1
593		12	max	320.999	3	229.202	3	77.718	1	0	3	-.004	15	.492	3
594			min	-76.017	10	-576.178	1	2.779	15	0	1	-.121	1	-1.153	1
595		13	max	321.431	3	228.015	3	77.718	1	0	3	-.003	15	.35	3
596			min	-75.537	10	-577.761	1	2.779	15	0	1	-.073	1	-.794	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	321.864	3	226.828	3	77.718	1	0	3	0	15	.209	3
598		min	-75.057	10	-579.344	1	2.779	15	0	1	-.025	1	-.435	1
599	15	max	322.296	3	225.64	3	77.718	1	0	3	.024	1	.068	3
600		min	-74.577	10	-580.927	1	2.779	15	0	1	0	15	-.075	1
601	16	max	322.728	3	224.453	3	77.718	1	0	3	.072	1	.286	1
602		min	-74.097	10	-582.51	1	2.779	15	0	1	.003	15	-.071	3
603	17	max	323.16	3	223.265	3	77.718	1	0	3	.12	1	.648	1
604		min	-73.617	10	-584.094	1	2.779	15	0	1	.004	15	-.21	3
605	18	max	-6.384	15	541.907	1	90.602	1	0	1	.174	1	.323	1
606		min	-174.634	1	-179.888	3	3.253	15	0	3	.006	15	-.103	3
607	19	max	-6.21	15	540.324	1	90.602	1	0	1	.23	1	.009	3
608		min	-174.058	1	-181.075	3	3.253	15	0	3	.008	15	-.013	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.188	1	.006	3	1.259e-2	1	NC	1	NC	1
2			min	0	15	-.027	3	-.003	2	-1.646e-3	3	NC	1	NC	1
3		2	max	0	1	.153	3	.035	1	1.389e-2	1	NC	5	NC	2
4			min	0	15	.002	15	0	10	-1.518e-3	3	1233.469	3	6617.917	1
5		3	max	0	1	.299	3	.081	1	1.519e-2	1	NC	5	NC	3
6			min	0	15	-.081	1	.003	15	-1.39e-3	3	680.923	3	2791.611	1
7		4	max	0	1	.389	3	.12	1	1.65e-2	1	NC	5	NC	3
8			min	0	15	-.142	1	.004	15	-1.262e-3	3	534.512	3	1874.903	1
9		5	max	0	1	.411	3	.139	1	1.78e-2	1	NC	5	NC	3
10			min	0	15	-.137	1	.005	15	-1.134e-3	3	507.759	3	1612.441	1
11		6	max	0	1	.367	3	.133	1	1.911e-2	1	NC	5	NC	3
12			min	0	15	-.068	1	.005	15	-1.005e-3	3	564.692	3	1683.746	1
13		7	max	0	1	.27	3	.104	1	2.041e-2	1	NC	5	NC	3
14			min	0	15	.002	15	.004	15	-8.773e-4	3	748.283	3	2167.189	1
15		8	max	0	1	.191	1	.059	1	2.171e-2	1	NC	1	NC	2
16			min	0	15	.006	15	0	10	-7.493e-4	3	1279.007	3	3828.69	1
17		9	max	0	1	.315	1	.019	3	2.302e-2	1	NC	5	NC	1
18			min	0	15	.009	15	-.006	10	-6.212e-4	3	1750.073	1	NC	1
19		10	max	0	1	.37	1	.018	3	2.432e-2	1	NC	3	NC	1
20			min	0	1	-.016	3	-.012	2	-4.931e-4	3	1219.893	1	NC	1
21		11	max	0	15	.315	1	.019	3	2.302e-2	1	NC	5	NC	1
22			min	0	1	.009	15	-.006	10	-6.212e-4	3	1750.073	1	NC	1
23		12	max	0	15	.191	1	.059	1	2.171e-2	1	NC	1	NC	2
24			min	0	1	.006	15	0	10	-7.493e-4	3	1279.007	3	3828.69	1
25		13	max	0	15	.27	3	.104	1	2.041e-2	1	NC	5	NC	3
26			min	0	1	.002	15	.004	15	-8.773e-4	3	748.283	3	2167.189	1
27		14	max	0	15	.367	3	.133	1	1.911e-2	1	NC	5	NC	3
28			min	0	1	-.068	1	.005	15	-1.005e-3	3	564.692	3	1683.746	1
29		15	max	0	15	.411	3	.139	1	1.78e-2	1	NC	5	NC	3
30			min	0	1	-.137	1	.005	15	-1.134e-3	3	507.759	3	1612.441	1
31		16	max	0	15	.389	3	.12	1	1.65e-2	1	NC	5	NC	3
32			min	0	1	-.142	1	.004	15	-1.262e-3	3	534.512	3	1874.903	1
33		17	max	0	15	.299	3	.081	1	1.519e-2	1	NC	5	NC	3
34			min	0	1	-.081	1	.003	15	-1.39e-3	3	680.923	3	2791.611	1
35		18	max	0	15	.153	3	.035	1	1.389e-2	1	NC	5	NC	2
36			min	0	1	.002	15	0	10	-1.518e-3	3	1233.469	3	6617.917	1
37		19	max	0	15	.188	1	.006	3	1.259e-2	1	NC	1	NC	1
38			min	0	1	-.027	3	-.003	2	-1.646e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.242	3	.005	3	7.616e-3	1	NC	1	NC	1
40			min	0	15	-.582	1	-.002	2	-3.747e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.442	3	.023	1	8.92e-3	1	NC	5	NC	1
42			min	0	15	-.903	1	0	10	-4.47e-3	3	691.407	1	NC	1
43		3	max	0	1	.615	3	.063	1	1.022e-2	1	NC	15	NC	3
44			min	0	15	-1.186	1	.002	15	-5.192e-3	3	367.44	1	3625.138	1
45		4	max	0	1	.742	3	.1	1	1.153e-2	1	9973.304	15	NC	3
46			min	0	15	-1.405	1	.004	15	-5.915e-3	3	269.864	1	2256.732	1
47		5	max	0	1	.813	3	.121	1	1.283e-2	1	8546.675	15	NC	3
48			min	0	15	-1.544	1	.004	15	-6.637e-3	3	230.732	1	1860.815	1
49		6	max	0	1	.827	3	.119	1	1.413e-2	1	8081.034	15	NC	3
50			min	0	15	-1.603	1	.004	15	-7.36e-3	3	217.473	1	1891.092	1
51		7	max	0	1	.795	3	.095	1	1.544e-2	1	8209.392	15	NC	3
52			min	0	15	-1.591	1	.004	15	-8.082e-3	3	220.008	1	2386.53	1
53		8	max	0	1	.732	3	.055	1	1.674e-2	1	8767.493	15	NC	2
54			min	0	15	-1.532	1	0	10	-8.805e-3	3	233.766	1	4142.646	1
55		9	max	0	1	.667	3	.017	3	1.804e-2	1	9534.545	15	NC	1
56			min	0	15	-1.46	1	-.005	10	-9.527e-3	3	252.9	1	NC	1
57		10	max	0	1	.636	3	.016	3	1.935e-2	1	9977.976	15	NC	1
58			min	0	1	-1.423	1	-.011	2	-1.025e-2	3	263.96	1	NC	1
59		11	max	0	15	.667	3	.017	3	1.804e-2	1	9534.545	15	NC	1
60			min	0	1	-1.46	1	-.005	10	-9.527e-3	3	252.9	1	NC	1
61		12	max	0	15	.732	3	.055	1	1.674e-2	1	8767.493	15	NC	2
62			min	0	1	-1.532	1	0	10	-8.805e-3	3	233.766	1	4142.646	1
63		13	max	0	15	.795	3	.095	1	1.544e-2	1	8209.392	15	NC	3
64			min	0	1	-1.591	1	.004	15	-8.082e-3	3	220.008	1	2386.53	1
65		14	max	0	15	.827	3	.119	1	1.413e-2	1	8081.034	15	NC	3
66			min	0	1	-1.603	1	.004	15	-7.36e-3	3	217.473	1	1891.092	1
67		15	max	0	15	.813	3	.121	1	1.283e-2	1	8546.675	15	NC	3
68			min	0	1	-1.544	1	.004	15	-6.637e-3	3	230.732	1	1860.815	1
69		16	max	0	15	.742	3	.1	1	1.153e-2	1	9973.304	15	NC	3
70			min	0	1	-1.405	1	.004	15	-5.915e-3	3	269.864	1	2256.732	1
71		17	max	0	15	.615	3	.063	1	1.022e-2	1	NC	15	NC	3
72			min	0	1	-1.186	1	.002	15	-5.192e-3	3	367.44	1	3625.138	1
73		18	max	0	15	.442	3	.023	1	8.92e-3	1	NC	5	NC	1
74			min	0	1	-.903	1	0	10	-4.47e-3	3	691.407	1	NC	1
75		19	max	0	15	.242	3	.005	3	7.616e-3	1	NC	1	NC	1
76			min	0	1	-.582	1	-.002	2	-3.747e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.248	3	.005	3	3.118e-3	3	NC	1	NC	1
78			min	0	1	-.581	1	-.002	2	-7.748e-3	1	NC	1	NC	1
79		2	max	0	15	.385	3	.023	1	3.716e-3	3	NC	5	NC	1
80			min	0	1	-.926	1	0	10	-9.083e-3	1	645.175	1	NC	1
81		3	max	0	15	.507	3	.063	1	4.314e-3	3	NC	15	NC	3
82			min	0	1	-1.227	1	.002	15	-1.042e-2	1	343.767	1	3605.856	1
83		4	max	0	15	.603	3	.1	1	4.912e-3	3	9986.586	15	NC	3
84			min	0	1	-1.457	1	.004	15	-1.175e-2	1	253.58	1	2247.232	1
85		5	max	0	15	.668	3	.121	1	5.511e-3	3	8559.293	15	NC	3
86			min	0	1	-1.599	1	.004	15	-1.309e-2	1	218.193	1	1853.575	1
87		6	max	0	15	.701	3	.119	1	6.109e-3	3	8094.579	15	NC	3
88			min	0	1	-1.651	1	.004	15	-1.442e-2	1	207.472	1	1883.282	1
89		7	max	0	15	.705	3	.095	1	6.707e-3	3	8225.326	15	NC	3
90			min	0	1	-1.627	1	.004	15	-1.576e-2	1	212.359	1	2374.09	1
91		8	max	0	15	.688	3	.056	1	7.305e-3	3	8787.371	15	NC	2
92			min	0	1	-1.551	1	0	10	-1.709e-2	1	228.926	1	4106.123	1
93		9	max	0	15	.665	3	.016	1	7.903e-3	3	9559.345	15	NC	1
94			min	0	1	-1.465	1	-.005	10	-1.843e-2	1	251.368	1	NC	1
95		10	max	0	1	.652	3	.015	3	8.501e-3	3	NC	15	NC	1
96			min	0	1	-1.421	1	-.01	2	-1.976e-2	1	264.372	1	NC	1
97		11	max	0	1	.665	3	.016	1	7.903e-3	3	9559.345	15	NC	1





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.465	1	-.005	10	-1.843e-2	1	251.368	1	NC	1
99		12	max	0	1	.688	3	.056	1	7.305e-3	3	8787.371	15	NC	2
100			min	0	15	-1.551	1	0	10	-1.709e-2	1	228.926	1	4106.123	1
101		13	max	0	1	.705	3	.095	1	6.707e-3	3	8225.326	15	NC	3
102			min	0	15	-1.627	1	.004	15	-1.576e-2	1	212.359	1	2374.09	1
103		14	max	0	1	.701	3	.119	1	6.109e-3	3	8094.579	15	NC	3
104			min	0	15	-1.651	1	.004	15	-1.442e-2	1	207.472	1	1883.282	1
105		15	max	0	1	.668	3	.121	1	5.511e-3	3	8559.293	15	NC	3
106			min	0	15	-1.599	1	.004	15	-1.309e-2	1	218.193	1	1853.575	1
107		16	max	0	1	.603	3	.1	1	4.912e-3	3	9986.586	15	NC	3
108			min	0	15	-1.457	1	.004	15	-1.175e-2	1	253.58	1	2247.232	1
109		17	max	0	1	.507	3	.063	1	4.314e-3	3	NC	15	NC	3
110			min	0	15	-1.227	1	.002	15	-1.042e-2	1	343.767	1	3605.856	1
111		18	max	0	1	.385	3	.023	1	3.716e-3	3	NC	5	NC	1
112			min	0	15	-.926	1	0	10	-9.083e-3	1	645.175	1	NC	1
113		19	max	0	1	.248	3	.005	3	3.118e-3	3	NC	1	NC	1
114			min	0	15	-.581	1	-.002	2	-7.748e-3	1	NC	1	NC	1
115	M16	1	max	0	15	.183	1	.004	3	5.706e-3	3	NC	1	NC	1
116			min	0	1	-.086	3	-.002	2	-1.189e-2	1	NC	1	NC	1
117		2	max	0	15	.015	9	.034	1	6.477e-3	3	NC	5	NC	2
118			min	0	1	-.034	3	.001	15	-1.303e-2	1	1260.606	1	6695.502	1
119		3	max	0	15	.005	3	.08	1	7.249e-3	3	NC	5	NC	3
120			min	0	1	-.144	2	.003	15	-1.417e-2	1	707.066	1	2807.417	1
121		4	max	0	15	.023	3	.119	1	8.021e-3	3	NC	5	NC	3
122			min	0	1	-.215	2	.004	15	-1.531e-2	1	571.947	1	1878.914	1
123		5	max	0	15	.015	3	.139	1	8.793e-3	3	NC	5	NC	3
124			min	0	1	-.217	2	.005	15	-1.645e-2	1	573.466	1	1611.034	1
125		6	max	0	15	0	15	.134	1	9.565e-3	3	NC	5	NC	3
126			min	0	1	-.155	2	.005	15	-1.759e-2	1	708.533	1	1676.174	1
127		7	max	0	15	.021	9	.105	1	1.034e-2	3	NC	3	NC	3
128			min	0	1	-.066	3	.004	15	-1.873e-2	1	1195.707	2	2144.214	1
129		8	max	0	15	.156	1	.061	1	1.111e-2	3	NC	4	NC	2
130			min	0	1	-.123	3	0	10	-1.986e-2	1	4634.162	2	3728.991	1
131		9	max	0	15	.294	1	.017	1	1.188e-2	3	NC	5	NC	1
132			min	0	1	-.173	3	-.004	10	-2.1e-2	1	1997.627	1	NC	1
133		10	max	0	1	.356	1	.013	3	1.265e-2	3	NC	5	NC	1
134			min	0	1	-.194	3	-.009	2	-2.214e-2	1	1285.22	1	NC	1
135		11	max	0	1	.294	1	.017	1	1.188e-2	3	NC	5	NC	1
136			min	0	15	-.173	3	-.004	10	-2.1e-2	1	1997.627	1	NC	1
137		12	max	0	1	.156	1	.061	1	1.111e-2	3	NC	4	NC	2
138			min	0	15	-.123	3	0	10	-1.986e-2	1	4634.162	2	3728.991	1
139		13	max	0	1	.021	9	.105	1	1.034e-2	3	NC	3	NC	3
140			min	0	15	-.066	3	.004	15	-1.873e-2	1	1195.707	2	2144.214	1
141		14	max	0	1	0	15	.134	1	9.565e-3	3	NC	5	NC	3
142			min	0	15	-.155	2	.005	15	-1.759e-2	1	708.533	1	1676.174	1
143		15	max	0	1	.015	3	.139	1	8.793e-3	3	NC	5	NC	3
144			min	0	15	-.217	2	.005	15	-1.645e-2	1	573.466	1	1611.034	1
145		16	max	0	1	.023	3	.119	1	8.021e-3	3	NC	5	NC	3
146			min	0	15	-.215	2	.004	15	-1.531e-2	1	571.947	1	1878.914	1
147		17	max	0	1	.005	3	.08	1	7.249e-3	3	NC	5	NC	3
148			min	0	15	-.144	2	.003	15	-1.417e-2	1	707.066	1	2807.417	1
149		18	max	0	1	.015	9	.034	1	6.477e-3	3	NC	5	NC	2
150			min	0	15	-.034	3	.001	15	-1.303e-2	1	1260.606	1	6695.502	1
151		19	max	0	1	.183	1	.004	3	5.706e-3	3	NC	1	NC	1
152			min	0	15	-.086	3	-.002	2	-1.189e-2	1	NC	1	NC	1
153	M2	1	max	.006	1	.005	2	.011	1	-8.585e-6	15	NC	1	NC	2
154			min	-.005	3	-.008	3	0	15	-2.399e-4	1	NC	1	5707.969	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	1	.004	2	.01	1	-8.041e-6	15	NC	1	NC	2
156		min	-.005	3	-.008	3	0	15	-2.247e-4	1	NC	1	6223.658	1
157	3	max	.006	1	.003	2	.009	1	-7.497e-6	15	NC	1	NC	2
158		min	-.004	3	-.008	3	0	15	-2.095e-4	1	NC	1	6837.789	1
159	4	max	.005	1	.002	2	.008	1	-6.953e-6	15	NC	1	NC	2
160		min	-.004	3	-.008	3	0	15	-1.943e-4	1	NC	1	7576.338	1
161	5	max	.005	1	.001	2	.007	1	-6.409e-6	15	NC	1	NC	2
162		min	-.004	3	-.007	3	0	15	-1.791e-4	1	NC	1	8474.715	1
163	6	max	.005	1	0	2	.006	1	-5.866e-6	15	NC	1	NC	2
164		min	-.004	3	-.007	3	0	15	-1.639e-4	1	NC	1	9582.203	1
165	7	max	.004	1	0	2	.006	1	-5.322e-6	15	NC	1	NC	1
166		min	-.003	3	-.007	3	0	15	-1.486e-4	1	NC	1	NC	1
167	8	max	.004	1	0	2	.005	1	-4.778e-6	15	NC	1	NC	1
168		min	-.003	3	-.007	3	0	15	-1.334e-4	1	NC	1	NC	1
169	9	max	.004	1	-.001	2	.004	1	-4.234e-6	15	NC	1	NC	1
170		min	-.003	3	-.006	3	0	15	-1.182e-4	1	NC	1	NC	1
171	10	max	.003	1	-.001	15	.003	1	-3.691e-6	15	NC	1	NC	1
172		min	-.003	3	-.006	3	0	15	-1.03e-4	1	NC	1	NC	1
173	11	max	.003	1	-.001	15	.003	1	-3.147e-6	15	NC	1	NC	1
174		min	-.002	3	-.006	3	0	15	-8.776e-5	1	NC	1	NC	1
175	12	max	.002	1	-.001	15	.002	1	-2.603e-6	15	NC	1	NC	1
176		min	-.002	3	-.005	3	0	15	-7.255e-5	1	NC	1	NC	1
177	13	max	.002	1	-.001	15	.002	1	-2.059e-6	15	NC	1	NC	1
178		min	-.002	3	-.005	3	0	15	-5.733e-5	1	NC	1	NC	1
179	14	max	.002	1	0	15	.001	1	-1.515e-6	15	NC	1	NC	1
180		min	-.001	3	-.004	4	0	15	-4.211e-5	1	NC	1	NC	1
181	15	max	.001	1	0	15	0	1	-9.716e-7	15	NC	1	NC	1
182		min	-.001	3	-.004	4	0	15	-2.689e-5	1	NC	1	NC	1
183	16	max	.001	1	0	15	0	1	-4.279e-7	15	NC	1	NC	1
184		min	0	3	-.003	4	0	15	-1.167e-5	1	NC	1	NC	1
185	17	max	0	1	0	15	0	1	3.547e-6	1	NC	1	NC	1
186		min	0	3	-.002	4	0	15	-2.329e-7	3	NC	1	NC	1
187	18	max	0	1	0	15	0	1	1.877e-5	1	NC	1	NC	1
188		min	0	3	-.001	4	0	15	6.099e-7	12	NC	1	NC	1
189	19	max	0	1	0	1	0	1	3.398e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	1.203e-6	15	NC	1	NC	1
191	M3	1	max	0	0	1	0	1	-3.667e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-1.033e-5	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	2.012e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	7.202e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	5.057e-5	1	NC	1	NC	1
196		min	0	2	-.005	4	0	15	1.807e-6	15	NC	1	NC	1
197	4	max	0	3	-.002	15	0	1	8.102e-5	1	NC	1	NC	1
198		min	0	2	-.008	4	0	15	2.894e-6	15	NC	1	NC	1
199	5	max	.001	3	-.003	15	0	1	1.115e-4	1	NC	1	NC	1
200		min	0	2	-.011	4	0	15	3.981e-6	15	9251.258	4	NC	1
201	6	max	.001	3	-.003	15	.001	1	1.419e-4	1	NC	2	NC	1
202		min	0	2	-.014	4	0	15	5.068e-6	15	7426.89	4	NC	1
203	7	max	.002	3	-.004	15	.001	1	1.724e-4	1	NC	5	NC	1
204		min	-.001	2	-.016	4	0	15	6.155e-6	15	6332.129	4	NC	1
205	8	max	.002	3	-.004	15	.002	1	2.028e-4	1	NC	5	NC	1
206		min	-.001	2	-.018	4	0	15	7.242e-6	15	5656.331	4	NC	1
207	9	max	.002	3	-.005	15	.002	1	2.333e-4	1	NC	5	NC	1
208		min	-.001	2	-.02	4	0	15	8.328e-6	15	5253.609	4	NC	1
209	10	max	.002	3	-.005	15	.003	1	2.637e-4	1	NC	5	NC	1
210		min	-.002	2	-.021	4	0	15	9.415e-6	15	5052.851	4	NC	1
211	11	max	.003	3	-.005	15	.003	1	2.942e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.002	2	-.021	4	0	15	1.05e-5	15	5024.134	4	NC	1
213		max	.003	3	-.005	15	.004	1	3.246e-4	1	NC	5	NC	1
214		min	-.002	2	-.02	4	0	15	1.159e-5	15	5167.187	4	NC	1
215		max	.003	3	-.004	15	.004	1	3.551e-4	1	NC	5	NC	1
216		min	-.002	2	-.019	4	0	15	1.268e-5	15	5512.726	4	NC	1
217		max	.003	3	-.004	15	.005	1	3.855e-4	1	NC	5	NC	1
218		min	-.002	2	-.017	4	0	15	1.376e-5	15	6138.689	4	NC	1
219		max	.004	3	-.003	15	.006	1	4.16e-4	1	NC	3	NC	1
220		min	-.003	2	-.015	4	0	15	1.485e-5	15	7219.056	4	NC	1
221		max	.004	3	-.003	15	.007	1	4.464e-4	1	NC	1	NC	1
222		min	-.003	2	-.012	4	0	15	1.594e-5	15	9176.137	4	NC	1
223		max	.004	3	-.002	15	.008	1	4.769e-4	1	NC	1	NC	1
224		min	-.003	2	-.008	4	0	15	1.702e-5	15	NC	1	NC	1
225		max	.004	3	-.001	15	.009	1	5.073e-4	1	NC	1	NC	1
226		min	-.003	2	-.006	1	0	15	1.811e-5	15	NC	1	NC	1
227		max	.005	3	0	15	.01	1	5.378e-4	1	NC	1	NC	1
228		min	-.003	2	-.003	1	0	15	1.92e-5	15	NC	1	NC	1
229	M4	max	.003	1	.003	2	0	15	8.135e-5	1	NC	1	NC	3
230		min	0	3	-.005	3	-.01	1	2.926e-6	15	NC	1	2505.909	1
231		max	.003	1	.003	2	0	15	8.135e-5	1	NC	1	NC	3
232		min	0	3	-.004	3	-.009	1	2.926e-6	15	NC	1	2725.572	1
233		max	.002	1	.003	2	0	15	8.135e-5	1	NC	1	NC	3
234		min	0	3	-.004	3	-.008	1	2.926e-6	15	NC	1	2986.972	1
235		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	3
236		min	0	3	-.004	3	-.008	1	2.926e-6	15	NC	1	3300.957	1
237		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	3
238		min	0	3	-.004	3	-.007	1	2.926e-6	15	NC	1	3682.269	1
239		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	2
240		min	0	3	-.003	3	-.006	1	2.926e-6	15	NC	1	4151.351	1
241		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	2
242		min	0	3	-.003	3	-.005	1	2.926e-6	15	NC	1	4737.226	1
243		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	2
244		min	0	3	-.003	3	-.005	1	2.926e-6	15	NC	1	5482.218	1
245		max	.002	1	.002	2	0	15	8.135e-5	1	NC	1	NC	2
246		min	0	3	-.003	3	-.004	1	2.926e-6	15	NC	1	6450.047	1
247		max	.001	1	.001	2	0	15	8.135e-5	1	NC	1	NC	2
248		min	0	3	-.002	3	-.003	1	2.926e-6	15	NC	1	7740.326	1
249		max	.001	1	.001	2	0	15	8.135e-5	1	NC	1	NC	2
250		min	0	3	-.002	3	-.003	1	2.926e-6	15	NC	1	9516.006	1
251		max	.001	1	.001	2	0	15	8.135e-5	1	NC	1	NC	1
252		min	0	3	-.002	3	-.002	1	2.926e-6	15	NC	1	NC	1
253		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	-.002	1	2.926e-6	15	NC	1	NC	1
255		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
256		min	0	3	-.001	3	-.001	1	2.926e-6	15	NC	1	NC	1
257		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	2.926e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
260		min	0	3	0	3	0	1	2.926e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	2.926e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	8.135e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.926e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	8.135e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.926e-6	15	NC	1	NC	1
267	M6	max	.02	1	.019	2	0	1	0	1	NC	3	NC	1
268		min	-.016	3	-.025	3	0	1	0	1	3273.275	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.019	1	.017	2	0	1	0	1	NC	3	NC	1
270		min	-.015	3	-.023	3	0	1	0	1	3612.166	2	NC	1
271	3	max	.018	1	.015	2	0	1	0	1	NC	3	NC	1
272		min	-.014	3	-.022	3	0	1	0	1	4025.771	2	NC	1
273	4	max	.017	1	.013	2	0	1	0	1	NC	3	NC	1
274		min	-.014	3	-.021	3	0	1	0	1	4537.294	2	NC	1
275	5	max	.016	1	.012	2	0	1	0	1	NC	3	NC	1
276		min	-.013	3	-.02	3	0	1	0	1	5180.036	2	NC	1
277	6	max	.014	1	.01	2	0	1	0	1	NC	3	NC	1
278		min	-.012	3	-.018	3	0	1	0	1	6003.233	2	NC	1
279	7	max	.013	1	.009	2	0	1	0	1	NC	1	NC	1
280		min	-.011	3	-.017	3	0	1	0	1	7082.304	2	NC	1
281	8	max	.012	1	.007	2	0	1	0	1	NC	1	NC	1
282		min	-.01	3	-.016	3	0	1	0	1	8537.84	2	NC	1
283	9	max	.011	1	.006	2	0	1	0	1	NC	1	NC	1
284		min	-.009	3	-.015	3	0	1	0	1	NC	1	NC	1
285	10	max	.01	1	.004	2	0	1	0	1	NC	1	NC	1
286		min	-.008	3	-.013	3	0	1	0	1	NC	1	NC	1
287	11	max	.009	1	.003	2	0	1	0	1	NC	1	NC	1
288		min	-.007	3	-.012	3	0	1	0	1	NC	1	NC	1
289	12	max	.008	1	.002	2	0	1	0	1	NC	1	NC	1
290		min	-.006	3	-.011	3	0	1	0	1	NC	1	NC	1
291	13	max	.007	1	.001	2	0	1	0	1	NC	1	NC	1
292		min	-.005	3	-.009	3	0	1	0	1	NC	1	NC	1
293	14	max	.006	1	0	2	0	1	0	1	NC	1	NC	1
294		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	0	1	0	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.001	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.002	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.002	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.003	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.003	2	-.011	4	0	1	0	1	9547.324	4	NC	1
315	6	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.004	2	-.014	4	0	1	0	1	7640.779	4	NC	1
317	7	max	.005	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.004	2	-.016	4	0	1	0	1	6498.385	4	NC	1
319	8	max	.005	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.005	2	-.018	4	0	1	0	1	5793.202	4	NC	1
321	9	max	.006	3	-.005	15	0	1	0	1	NC	5	NC	1
322		min	-.006	2	-.02	4	0	1	0	1	5371.859	4	NC	1
323	10	max	.007	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.007	2	-.021	4	0	1	0	1	5159.502	4	NC	1
325	11	max	.008	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.007	2	-.021	4	0	1	0	1	5124.301	4	NC	1
327		12	max	.009	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.008	2	-.02	4	0	1	0	1	5265.15	4	NC	1
329		13	max	.009	3	-.004	15	0	1	0	1	NC	5	NC	1
330			min	-.009	2	-.019	4	0	1	0	1	5612.739	4	NC	1
331		14	max	.01	3	-.004	15	0	1	0	1	NC	5	NC	1
332			min	-.01	2	-.017	4	0	1	0	1	6245.91	4	NC	1
333		15	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
334			min	-.01	2	-.015	4	0	1	0	1	7341.187	4	NC	1
335		16	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.011	2	-.012	4	0	1	0	1	9327.432	4	NC	1
337		17	max	.012	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.012	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.013	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.013	2	-.008	1	0	1	0	1	NC	1	NC	1
341		19	max	.014	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.013	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.012	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.014	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.011	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.013	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	1	.011	2	0	1	0	1	NC	1	NC	1
348			min	-.001	3	-.013	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.01	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.012	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.009	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	1	.009	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.008	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.007	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.006	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	1	.004	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.002	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	1	.005	2	0	15	2.399e-4	1	NC	1	NC	2
382			min	-.005	3	-.008	3	-.011	1	8.585e-6	15	NC	1	5707.969	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.006	1	.004	2	0	15	2.247e-4	1	NC	1	NC	2
384		min	-.005	3	-.008	3	-.01	1	8.041e-6	15	NC	1	6223.658	1
385	3	max	.006	1	.003	2	0	15	2.095e-4	1	NC	1	NC	2
386		min	-.004	3	-.008	3	-.009	1	7.497e-6	15	NC	1	6837.789	1
387	4	max	.005	1	.002	2	0	15	1.943e-4	1	NC	1	NC	2
388		min	-.004	3	-.008	3	-.008	1	6.953e-6	15	NC	1	7576.338	1
389	5	max	.005	1	.001	2	0	15	1.791e-4	1	NC	1	NC	2
390		min	-.004	3	-.007	3	-.007	1	6.409e-6	15	NC	1	8474.715	1
391	6	max	.005	1	0	2	0	15	1.639e-4	1	NC	1	NC	2
392		min	-.004	3	-.007	3	-.006	1	5.866e-6	15	NC	1	9582.203	1
393	7	max	.004	1	0	2	0	15	1.486e-4	1	NC	1	NC	1
394		min	-.003	3	-.007	3	-.006	1	5.322e-6	15	NC	1	NC	1
395	8	max	.004	1	0	2	0	15	1.334e-4	1	NC	1	NC	1
396		min	-.003	3	-.007	3	-.005	1	4.778e-6	15	NC	1	NC	1
397	9	max	.004	1	-.001	2	0	15	1.182e-4	1	NC	1	NC	1
398		min	-.003	3	-.006	3	-.004	1	4.234e-6	15	NC	1	NC	1
399	10	max	.003	1	-.001	15	0	15	1.03e-4	1	NC	1	NC	1
400		min	-.003	3	-.006	3	-.003	1	3.691e-6	15	NC	1	NC	1
401	11	max	.003	1	-.001	15	0	15	8.776e-5	1	NC	1	NC	1
402		min	-.002	3	-.006	3	-.003	1	3.147e-6	15	NC	1	NC	1
403	12	max	.002	1	-.001	15	0	15	7.255e-5	1	NC	1	NC	1
404		min	-.002	3	-.005	3	-.002	1	2.603e-6	15	NC	1	NC	1
405	13	max	.002	1	-.001	15	0	15	5.733e-5	1	NC	1	NC	1
406		min	-.002	3	-.005	3	-.002	1	2.059e-6	15	NC	1	NC	1
407	14	max	.002	1	0	15	0	15	4.211e-5	1	NC	1	NC	1
408		min	-.001	3	-.004	4	-.001	1	1.515e-6	15	NC	1	NC	1
409	15	max	.001	1	0	15	0	15	2.689e-5	1	NC	1	NC	1
410		min	-.001	3	-.004	4	0	1	9.716e-7	15	NC	1	NC	1
411	16	max	.001	1	0	15	0	15	1.167e-5	1	NC	1	NC	1
412		min	0	3	-.003	4	0	1	4.279e-7	15	NC	1	NC	1
413	17	max	0	1	0	15	0	15	2.329e-7	3	NC	1	NC	1
414		min	0	3	-.002	4	0	1	-3.547e-6	1	NC	1	NC	1
415	18	max	0	1	0	15	0	15	-6.099e-7	12	NC	1	NC	1
416		min	0	3	-.001	4	0	1	-1.877e-5	1	NC	1	NC	1
417	19	max	0	1	0	1	0	1	-1.203e-6	15	NC	1	NC	1
418		min	0	1	0	1	0	1	-3.398e-5	1	NC	1	NC	1
419	M11	1	max	0	0	1	0	1	1.033e-5	1	NC	1	NC	1
420		min	0	1	0	1	0	1	3.667e-7	15	NC	1	NC	1
421	2	max	0	3	0	15	0	15	-7.202e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	1	-2.012e-5	1	NC	1	NC	1
423	3	max	0	3	-.001	15	0	15	-1.807e-6	15	NC	1	NC	1
424		min	0	2	-.005	4	0	1	-5.057e-5	1	NC	1	NC	1
425	4	max	0	3	-.002	15	0	15	-2.894e-6	15	NC	1	NC	1
426		min	0	2	-.008	4	0	1	-8.102e-5	1	NC	1	NC	1
427	5	max	.001	3	-.003	15	0	15	-3.981e-6	15	NC	1	NC	1
428		min	0	2	-.011	4	0	1	-1.115e-4	1	9251.258	4	NC	1
429	6	max	.001	3	-.003	15	0	15	-5.068e-6	15	NC	2	NC	1
430		min	0	2	-.014	4	-.001	1	-1.419e-4	1	7426.89	4	NC	1
431	7	max	.002	3	-.004	15	0	15	-6.155e-6	15	NC	5	NC	1
432		min	-.001	2	-.016	4	-.001	1	-1.724e-4	1	6332.129	4	NC	1
433	8	max	.002	3	-.004	15	0	15	-7.242e-6	15	NC	5	NC	1
434		min	-.001	2	-.018	4	-.002	1	-2.028e-4	1	5656.331	4	NC	1
435	9	max	.002	3	-.005	15	0	15	-8.328e-6	15	NC	5	NC	1
436		min	-.001	2	-.02	4	-.002	1	-2.333e-4	1	5253.609	4	NC	1
437	10	max	.002	3	-.005	15	0	15	-9.415e-6	15	NC	5	NC	1
438		min	-.002	2	-.021	4	-.003	1	-2.637e-4	1	5052.851	4	NC	1
439	11	max	.003	3	-.005	15	0	15	-1.05e-5	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.002	2	-.021	4	-.003	1	-2.942e-4	1	5024.134	4	NC	1
441		max	.003	3	-.005	15	0	15	-1.159e-5	15	NC	5	NC	1
442		min	-.002	2	-.02	4	-.004	1	-3.246e-4	1	5167.187	4	NC	1
443		max	.003	3	-.004	15	0	15	-1.268e-5	15	NC	5	NC	1
444		min	-.002	2	-.019	4	-.004	1	-3.551e-4	1	5512.726	4	NC	1
445		max	.003	3	-.004	15	0	15	-1.376e-5	15	NC	5	NC	1
446		min	-.002	2	-.017	4	-.005	1	-3.855e-4	1	6138.689	4	NC	1
447		max	.004	3	-.003	15	0	15	-1.485e-5	15	NC	3	NC	1
448		min	-.003	2	-.015	4	-.006	1	-4.16e-4	1	7219.056	4	NC	1
449		max	.004	3	-.003	15	0	15	-1.594e-5	15	NC	1	NC	1
450		min	-.003	2	-.012	4	-.007	1	-4.464e-4	1	9176.137	4	NC	1
451		max	.004	3	-.002	15	0	15	-1.702e-5	15	NC	1	NC	1
452		min	-.003	2	-.008	4	-.008	1	-4.769e-4	1	NC	1	NC	1
453		max	.004	3	-.001	15	0	15	-1.811e-5	15	NC	1	NC	1
454		min	-.003	2	-.006	1	-.009	1	-5.073e-4	1	NC	1	NC	1
455		max	.005	3	0	15	0	15	-1.92e-5	15	NC	1	NC	1
456		min	-.003	2	-.003	1	-.01	1	-5.378e-4	1	NC	1	NC	1
457	M12	max	.003	1	.003	2	.01	1	-2.926e-6	15	NC	1	NC	3
458		min	0	3	-.005	3	0	15	-8.135e-5	1	NC	1	2505.909	1
459		max	.003	1	.003	2	.009	1	-2.926e-6	15	NC	1	NC	3
460		min	0	3	-.004	3	0	15	-8.135e-5	1	NC	1	2725.572	1
461		max	.002	1	.003	2	.008	1	-2.926e-6	15	NC	1	NC	3
462		min	0	3	-.004	3	0	15	-8.135e-5	1	NC	1	2986.972	1
463		max	.002	1	.002	2	.008	1	-2.926e-6	15	NC	1	NC	3
464		min	0	3	-.004	3	0	15	-8.135e-5	1	NC	1	3300.957	1
465		max	.002	1	.002	2	.007	1	-2.926e-6	15	NC	1	NC	3
466		min	0	3	-.004	3	0	15	-8.135e-5	1	NC	1	3682.269	1
467		max	.002	1	.002	2	.006	1	-2.926e-6	15	NC	1	NC	2
468		min	0	3	-.003	3	0	15	-8.135e-5	1	NC	1	4151.351	1
469		max	.002	1	.002	2	.005	1	-2.926e-6	15	NC	1	NC	2
470		min	0	3	-.003	3	0	15	-8.135e-5	1	NC	1	4737.226	1
471		max	.002	1	.002	2	.005	1	-2.926e-6	15	NC	1	NC	2
472		min	0	3	-.003	3	0	15	-8.135e-5	1	NC	1	5482.218	1
473		max	.002	1	.002	2	.004	1	-2.926e-6	15	NC	1	NC	2
474		min	0	3	-.003	3	0	15	-8.135e-5	1	NC	1	6450.047	1
475		max	.001	1	.001	2	.003	1	-2.926e-6	15	NC	1	NC	2
476		min	0	3	-.002	3	0	15	-8.135e-5	1	NC	1	7740.326	1
477		max	.001	1	.001	2	.003	1	-2.926e-6	15	NC	1	NC	2
478		min	0	3	-.002	3	0	15	-8.135e-5	1	NC	1	9516.006	1
479		max	.001	1	.001	2	.002	1	-2.926e-6	15	NC	1	NC	1
480		min	0	3	-.002	3	0	15	-8.135e-5	1	NC	1	NC	1
481		max	0	1	0	2	.002	1	-2.926e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-8.135e-5	1	NC	1	NC	1
483		max	0	1	0	2	.001	1	-2.926e-6	15	NC	1	NC	1
484		min	0	3	-.001	3	0	15	-8.135e-5	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-2.926e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-8.135e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-2.926e-6	15	NC	1	NC	1
488		min	0	3	0	3	0	15	-8.135e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.926e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-8.135e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.926e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-8.135e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.926e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-8.135e-5	1	NC	1	NC	1
495	M1	max	.006	3	.188	1	0	1	1.282e-2	1	NC	1	NC	1
496		min	-.003	2	-.027	3	0	15	-1.514e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.006	3	.094	1	0	15	6.189e-3	1	NC	5	NC	1
498			min	-.003	2	-.013	3	-.008	1	-7.515e-3	3	1424.301	1	NC	1
499		3	max	.006	3	.008	3	0	15	3.807e-6	10	NC	5	NC	1
500			min	-.003	2	-.008	1	-.011	1	-2.377e-4	1	683.942	1	NC	1
501		4	max	.006	3	.044	3	0	15	4.737e-3	1	NC	15	NC	1
502			min	-.003	2	-.124	1	-.01	1	-3.15e-3	3	430.027	1	NC	1
503		5	max	.006	3	.09	3	0	15	9.711e-3	1	9445.433	15	NC	1
504			min	-.003	2	-.246	1	-.007	1	-6.22e-3	3	309.127	1	NC	1
505		6	max	.006	3	.14	3	0	15	1.469e-2	1	7464.651	15	NC	1
506			min	-.003	2	-.365	1	-.003	1	-9.291e-3	3	242.685	1	NC	1
507		7	max	.005	3	.189	3	0	1	1.966e-2	1	6295.223	15	NC	1
508			min	-.002	2	-.472	1	0	3	-1.236e-2	3	203.561	1	NC	1
509		8	max	.005	3	.229	3	0	1	2.463e-2	1	5603.284	15	NC	1
510			min	-.002	2	-.557	1	0	15	-1.543e-2	3	180.462	1	NC	1
511		9	max	.005	3	.255	3	0	15	2.699e-2	1	5241.379	15	NC	1
512			min	-.002	2	-.61	1	0	1	-1.566e-2	3	168.434	1	NC	1
513		10	max	.005	3	.265	3	0	1	2.76e-2	1	5130.81	15	NC	1
514			min	-.002	2	-.628	1	0	15	-1.399e-2	3	164.819	1	NC	1
515		11	max	.005	3	.259	3	0	1	2.82e-2	1	5241.198	15	NC	1
516			min	-.002	2	-.61	1	0	15	-1.233e-2	3	168.621	1	NC	1
517		12	max	.005	3	.237	3	0	15	2.649e-2	1	5602.902	15	NC	1
518			min	-.002	2	-.555	1	-.001	1	-1.049e-2	3	181.034	1	NC	1
519		13	max	.005	3	.202	3	0	15	2.133e-2	1	6294.559	15	NC	1
520			min	-.002	2	-.469	1	0	1	-8.388e-3	3	204.954	1	NC	1
521		14	max	.005	3	.157	3	.003	1	1.616e-2	1	7463.53	15	NC	1
522			min	-.002	2	-.361	1	0	15	-6.291e-3	3	245.656	1	NC	1
523		15	max	.004	3	.106	3	.006	1	1.099e-2	1	9443.503	15	NC	1
524			min	-.002	2	-.241	1	0	15	-4.193e-3	3	315.202	1	NC	1
525		16	max	.004	3	.053	3	.009	1	5.826e-3	1	NC	15	NC	1
526			min	-.002	2	-.119	1	0	15	-2.096e-3	3	442.691	1	NC	1
527		17	max	.004	3	.003	3	.01	1	6.589e-4	1	NC	5	NC	1
528			min	-.002	2	-.004	2	0	15	1.212e-6	3	712.161	1	NC	1
529		18	max	.004	3	.094	1	.007	1	7.669e-3	1	NC	5	NC	1
530			min	-.002	2	-.043	3	0	15	-2.161e-3	3	1495.505	1	NC	1
531		19	max	.004	3	.183	1	0	15	1.49e-2	1	NC	1	NC	1
532			min	-.002	2	-.086	3	0	1	-4.4e-3	3	NC	1	NC	1
533	M5	1	max	.018	3	.37	1	0	1	0	1	NC	1	NC	1
534			min	-.012	2	-.016	3	0	1	0	1	NC	1	NC	1
535		2	max	.018	3	.186	1	0	1	0	1	NC	5	NC	1
536			min	-.012	2	-.009	3	0	1	0	1	729.796	1	NC	1
537		3	max	.018	3	.025	3	0	1	0	1	NC	15	NC	1
538			min	-.012	2	-.027	1	0	1	0	1	338.469	1	NC	1
539		4	max	.018	3	.107	3	0	1	0	1	6941.944	15	NC	1
540			min	-.012	2	-.291	1	0	1	0	1	203.581	1	NC	1
541		5	max	.017	3	.224	3	0	1	0	1	4836.252	15	NC	1
542			min	-.012	2	-.582	1	0	1	0	1	141.206	1	NC	1
543		6	max	.017	3	.358	3	0	1	0	1	3710.78	15	NC	1
544			min	-.012	2	-.875	1	0	1	0	1	107.966	1	NC	1
545		7	max	.017	3	.49	3	0	1	0	1	3062.964	15	NC	1
546			min	-.011	2	-1.143	1	0	1	0	1	88.875	1	NC	1
547		8	max	.016	3	.601	3	0	1	0	1	2687.355	15	NC	1
548			min	-.011	2	-1.358	1	0	1	0	1	77.824	1	NC	1
549		9	max	.016	3	.673	3	0	1	0	1	2494.902	15	NC	1
550			min	-.011	2	-1.493	1	0	1	0	1	72.173	1	NC	1
551		10	max	.015	3	.7	3	0	1	0	1	2436.886	15	NC	1
552			min	-.011	2	-1.539	1	0	1	0	1	70.489	1	NC	1
553		11	max	.015	3	.683	3	0	1	0	1	2494.985	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.01	2	-1.493	1	0	1	0	1	72.265	1	NC	1
555		max	.015	3	.623	3	0	1	0	1	2687.553	15	NC	1
556		min	-.01	2	-1.354	1	0	1	0	1	78.131	1	NC	1
557		max	.014	3	.527	3	0	1	0	1	3063.371	15	NC	1
558		min	-.01	2	-1.134	1	0	1	0	1	89.68	1	NC	1
559		max	.014	3	.406	3	0	1	0	1	3711.577	15	NC	1
560		min	-.01	2	-.861	1	0	1	0	1	109.797	1	NC	1
561		max	.014	3	.271	3	0	1	0	1	4837.833	15	NC	1
562		min	-.01	2	-.564	1	0	1	0	1	145.23	1	NC	1
563		max	.013	3	.134	3	0	1	0	1	6945.268	15	NC	1
564		min	-.01	2	-.272	1	0	1	0	1	212.73	1	NC	1
565		max	.013	3	.008	3	0	1	0	1	NC	15	NC	1
566		min	-.009	2	-.014	1	0	1	0	1	361.061	1	NC	1
567		max	.013	3	.187	1	0	1	0	1	NC	5	NC	1
568		min	-.009	2	-.099	3	0	1	0	1	791.319	1	NC	1
569		max	.013	3	.356	1	0	1	0	1	NC	1	NC	1
570		min	-.009	2	-.194	3	0	1	0	1	NC	1	NC	1
571	M9	max	.006	3	.188	1	0	15	1.514e-2	3	NC	1	NC	1
572		min	-.003	2	-.027	3	0	1	-1.282e-2	1	NC	1	NC	1
573		max	.006	3	.094	1	.008	1	7.515e-3	3	NC	5	NC	1
574		min	-.003	2	-.013	3	0	15	-6.189e-3	1	1424.301	1	NC	1
575		max	.006	3	.008	3	.011	1	2.377e-4	1	NC	5	NC	1
576		min	-.003	2	-.008	1	0	15	-3.807e-6	10	683.942	1	NC	1
577		max	.006	3	.044	3	.01	1	3.15e-3	3	NC	15	NC	1
578		min	-.003	2	-.124	1	0	15	-4.737e-3	1	430.027	1	NC	1
579		max	.006	3	.09	3	.007	1	6.22e-3	3	9445.433	15	NC	1
580		min	-.003	2	-.246	1	0	15	-9.711e-3	1	309.127	1	NC	1
581		max	.006	3	.14	3	.003	1	9.291e-3	3	7464.651	15	NC	1
582		min	-.003	2	-.365	1	0	15	-1.469e-2	1	242.685	1	NC	1
583		max	.005	3	.189	3	0	3	1.236e-2	3	6295.223	15	NC	1
584		min	-.002	2	-.472	1	0	1	-1.966e-2	1	203.561	1	NC	1
585		max	.005	3	.229	3	0	15	1.543e-2	3	5603.284	15	NC	1
586		min	-.002	2	-.557	1	0	1	-2.463e-2	1	180.462	1	NC	1
587		max	.005	3	.255	3	0	1	1.566e-2	3	5241.379	15	NC	1
588		min	-.002	2	-.61	1	0	15	-2.699e-2	1	168.434	1	NC	1
589		max	.005	3	.265	3	0	15	1.399e-2	3	5130.81	15	NC	1
590		min	-.002	2	-.628	1	0	1	-2.76e-2	1	164.819	1	NC	1
591		max	.005	3	.259	3	0	15	1.233e-2	3	5241.198	15	NC	1
592		min	-.002	2	-.61	1	0	1	-2.82e-2	1	168.621	1	NC	1
593		max	.005	3	.237	3	.001	1	1.049e-2	3	5602.902	15	NC	1
594		min	-.002	2	-.555	1	0	15	-2.649e-2	1	181.034	1	NC	1
595		max	.005	3	.202	3	0	1	8.388e-3	3	6294.559	15	NC	1
596		min	-.002	2	-.469	1	0	15	-2.133e-2	1	204.954	1	NC	1
597		max	.005	3	.157	3	0	15	6.291e-3	3	7463.53	15	NC	1
598		min	-.002	2	-.361	1	-.003	1	-1.616e-2	1	245.656	1	NC	1
599		max	.004	3	.106	3	0	15	4.193e-3	3	9443.503	15	NC	1
600		min	-.002	2	-.241	1	-.006	1	-1.099e-2	1	315.202	1	NC	1
601		max	.004	3	.053	3	0	15	2.096e-3	3	NC	15	NC	1
602		min	-.002	2	-.119	1	-.009	1	-5.826e-3	1	442.691	1	NC	1
603		max	.004	3	.003	3	0	15	-1.212e-6	3	NC	5	NC	1
604		min	-.002	2	-.004	2	-.01	1	-6.589e-4	1	712.161	1	NC	1
605		max	.004	3	.094	1	0	15	2.161e-3	3	NC	5	NC	1
606		min	-.002	2	-.043	3	-.007	1	-7.669e-3	1	1495.505	1	NC	1
607		max	.004	3	.183	1	0	1	4.4e-3	3	NC	1	NC	1
608		min	-.002	2	-.086	3	0	15	-1.49e-2	1	NC	1	NC	1



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Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

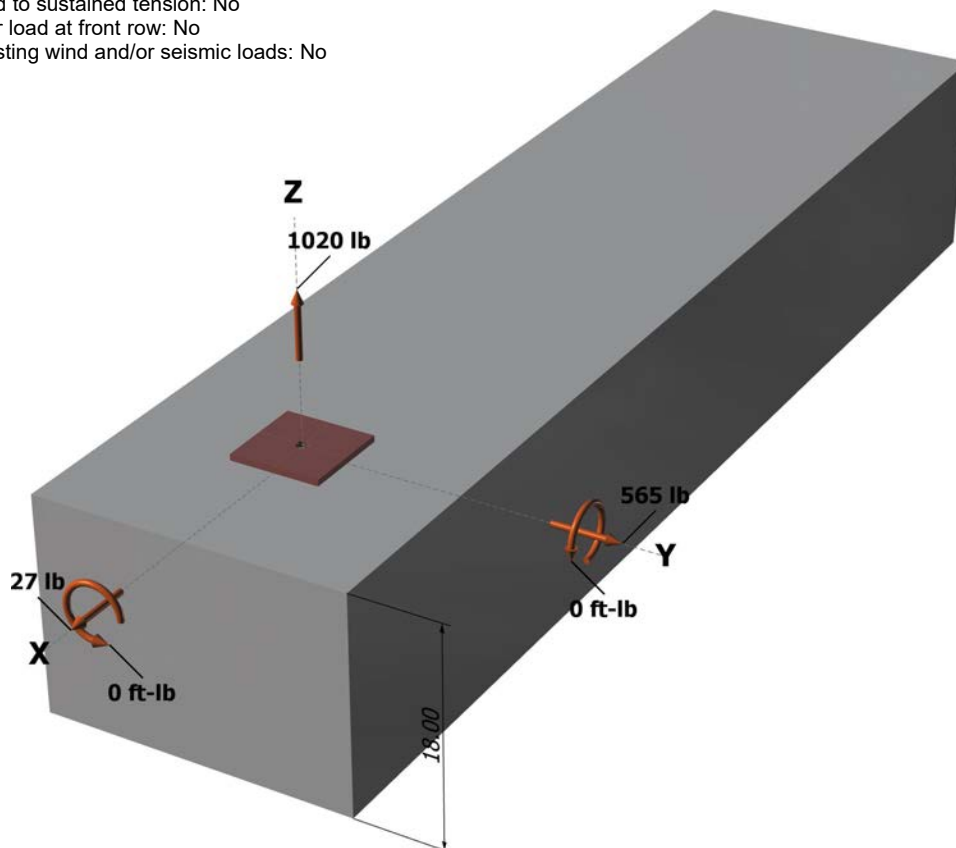
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

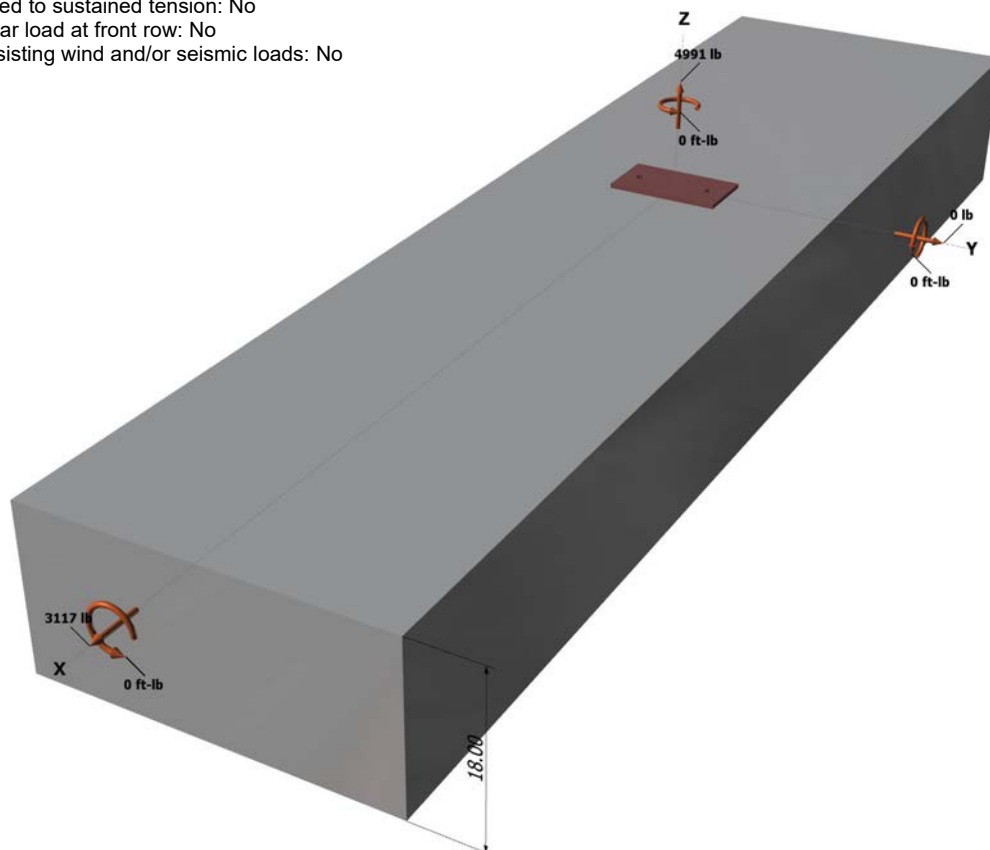
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

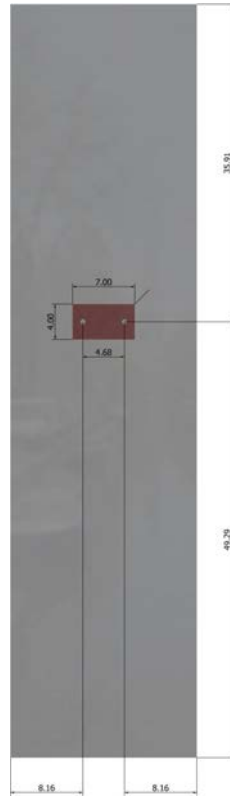
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.