

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ =	1.25
$S_{DS}$ =	1.67	$C_s$ =	0.8
$S_1$ =	1.00	$\rho$ =	1.3
$S_{D1}$ =	1.00	$\Omega$ =	1.25
$T_a$ =	0.06	$C_d$ =	1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

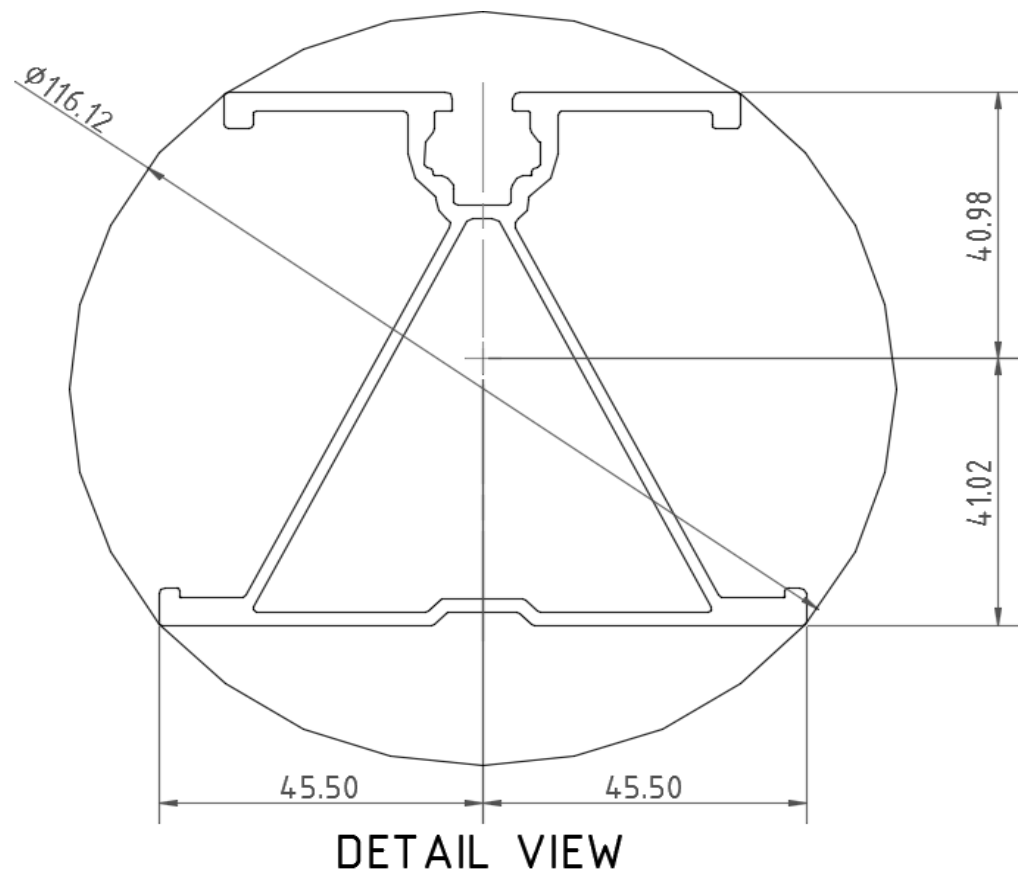
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	129 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.740 k-ft
$M_z$ =	0.390 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>96%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.651 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.697 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>78%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

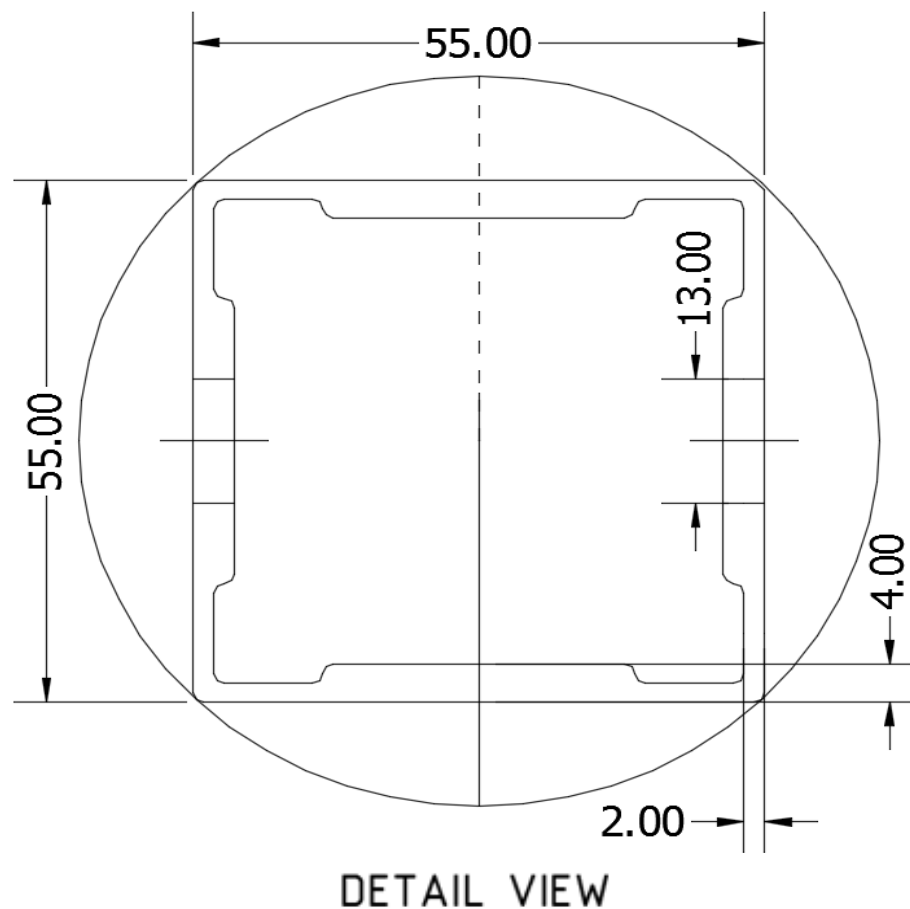
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.581 k-ft
$P_n$ =	0.635 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>44%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.085 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>29%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>70.83</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	10.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.008 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.913 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.365 k
Utilization =	<u>29%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

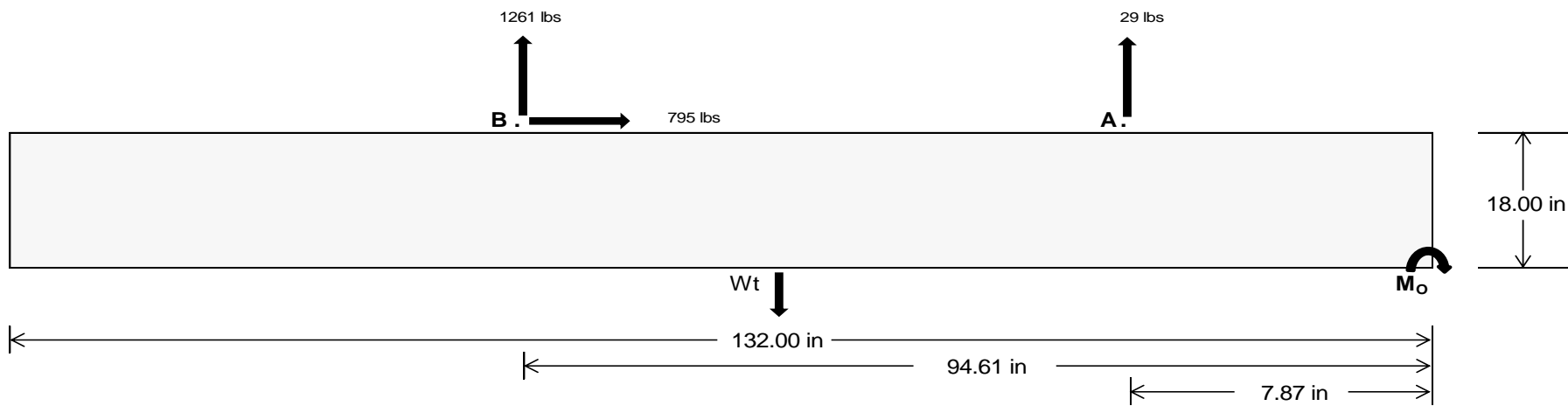
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>135.52</u>	<u>5258.31</u> k
Compressive Load =	<u>3479.92</u>	<u>4392.69</u> k
Lateral Load =	<u>401.84</u>	<u>3308.84</u> k
Moment (Weak Axis) =	<u>0.78</u>	<u>0.30</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 133843.2$  in-lbs  
Resisting Force Required = 2027.93 lbs  
S.F. = 1.67  
Weight Required = 3379.88 lbs  
Minimum Width = 27 in in  
Weight Provided = 5383.13 lbs

### Sliding

Force = 795.02 lbs  
Friction = 0.4  
Weight Required = 1987.54 lbs  
Resisting Weight = 5383.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 795.02 lbs  
Cohesion = 130 psf  
Area = 24.75 ft<sup>2</sup>  
Resisting = 2691.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 27in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 27in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 27in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.25 \text{ ft}) =$

Ballast Width	27 in	28 in	29 in	30 in
$P_{ftg}$	5383 lbs	5583 lbs	5782 lbs	5981 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in
$F_A$	1317 lbs	1317 lbs	1317 lbs	1317 lbs	1132 lbs	1132 lbs	1132 lbs	1132 lbs	1697 lbs	1697 lbs	1697 lbs	1697 lbs	-58 lbs	-58 lbs	-58 lbs	-58 lbs
$F_B$	1256 lbs	1256 lbs	1256 lbs	1256 lbs	1868 lbs	1868 lbs	1868 lbs	1868 lbs	2211 lbs	2211 lbs	2211 lbs	2211 lbs	-2522 lbs	-2522 lbs	-2522 lbs	-2522 lbs
$F_V$	202 lbs	202 lbs	202 lbs	202 lbs	1456 lbs	1456 lbs	1456 lbs	1456 lbs	1223 lbs	1223 lbs	1223 lbs	1223 lbs	-1590 lbs	-1590 lbs	-1590 lbs	-1590 lbs
$P_{total}$	7956 lbs	8155 lbs	8355 lbs	8554 lbs	8383 lbs	8583 lbs	8782 lbs	8981 lbs	9291 lbs	9491 lbs	9690 lbs	9889 lbs	649 lbs	769 lbs	889 lbs	1008 lbs
$M$	3687 lbs-ft	3687 lbs-ft	3687 lbs-ft	3687 lbs-ft	3216 lbs-ft	3216 lbs-ft	3216 lbs-ft	3216 lbs-ft	4788 lbs-ft	4788 lbs-ft	4788 lbs-ft	4788 lbs-ft	3344 lbs-ft	3344 lbs-ft	3344 lbs-ft	3344 lbs-ft
$e$	0.46 ft	0.45 ft	0.44 ft	0.43 ft	0.38 ft	0.37 ft	0.37 ft	0.36 ft	0.52 ft	0.50 ft	0.49 ft	0.48 ft	5.15 ft	4.35 ft	3.76 ft	3.32 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	240.2 psf	239.4 psf	238.6 psf	237.9 psf	267.9 psf	266.1 psf	264.4 psf	262.8 psf	269.9 psf	268.0 psf	266.3 psf	264.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	402.7 psf	396.1 psf	389.9 psf	384.2 psf	409.6 psf	402.7 psf	396.3 psf	390.4 psf	480.9 psf	471.5 psf	462.7 psf	454.6 psf	549.9 psf	190.9 psf	141.2 psf	123.2 psf

Maximum Bearing Pressure = 550 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 27in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

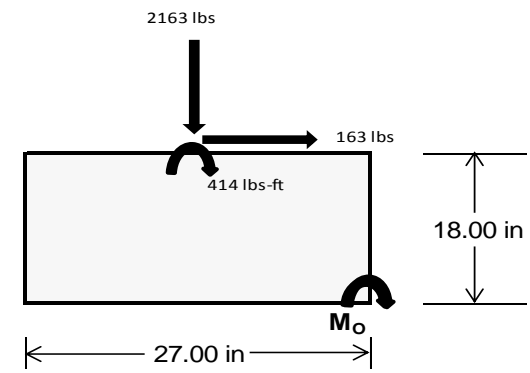
### Overturning Check

$M_o = 1774.4 \text{ ft-lbs}$   
 Resisting Force Required = 1577.22 lbs  
 S.F. = 1.67  
 Weight Required = 2628.69 lbs  
 Minimum Width = 27 in  
 Weight Provided = 5383.13 lbs

*A minimum 132in long x 27in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	27 in			27 in			27 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_y$	310 lbs	673 lbs	219 lbs	810 lbs	2163 lbs	740 lbs	122 lbs	197 lbs	32 lbs
$F_v$	226 lbs	222 lbs	231 lbs	166 lbs	163 lbs	180 lbs	227 lbs	223 lbs	229 lbs
$P_{total}$	6974 lbs	7337 lbs	6883 lbs	7154 lbs	8507 lbs	7084 lbs	2071 lbs	2145 lbs	1981 lbs
$M$	882 lbs-ft	871 lbs-ft	897 lbs-ft	656 lbs-ft	659 lbs-ft	702 lbs-ft	881 lbs-ft	869 lbs-ft	886 lbs-ft
$e$	0.13 ft	0.12 ft	0.13 ft	0.09 ft	0.08 ft	0.10 ft	0.43 ft	0.40 ft	0.45 ft
$L/6$	0.38 ft	0.38 ft	0.38 ft	0.38 ft	0.38 ft	0.38 ft	0.38 ft	0.38 ft	0.38 ft
$f_{min}$	186.8 psf	202.6 psf	181.5 psf	218.4 psf	272.7 psf	210.6 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	376.8 psf	390.3 psf	374.7 psf	359.7 psf	414.7 psf	361.9 psf	179.4 psf	180.6 psf	177.2 psf



Maximum Bearing Pressure = 415 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 27in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 27in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

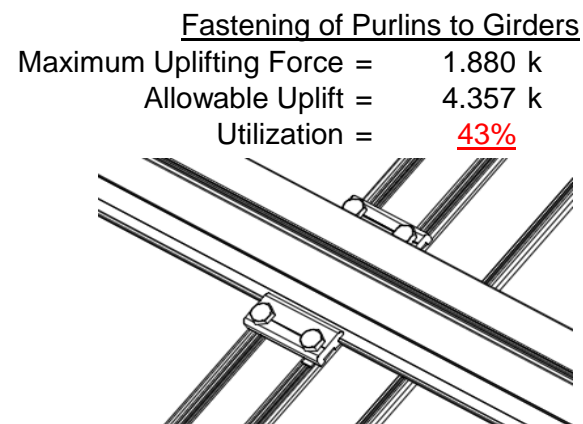
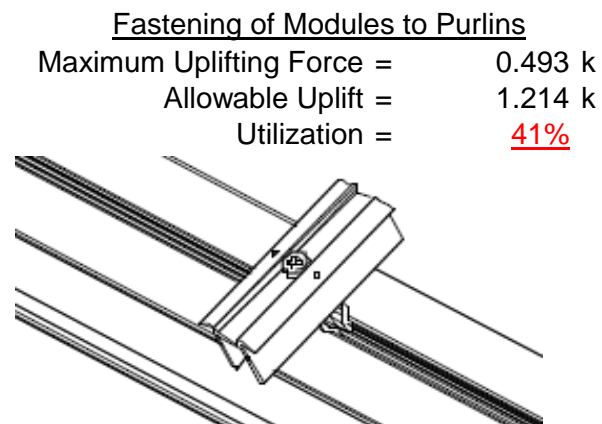
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

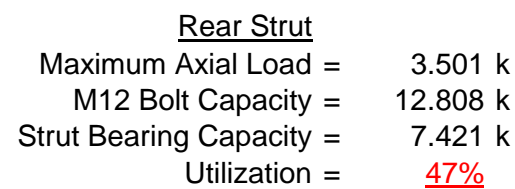
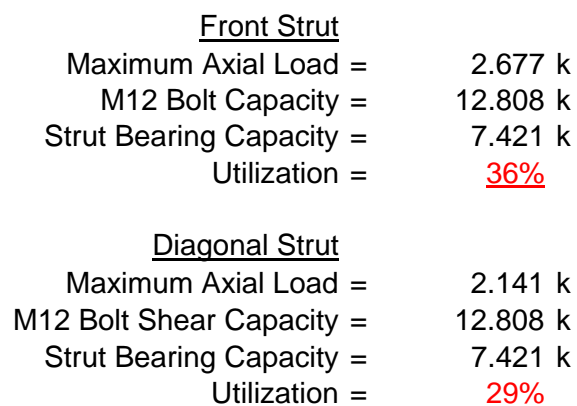
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



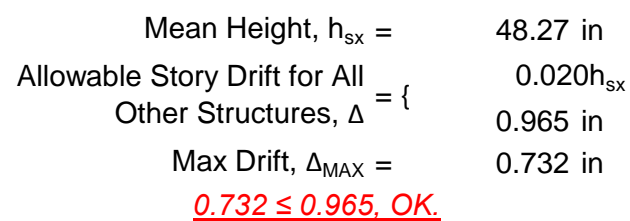
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 129 \text{ in}$$

$$J = 0.432$$

$$356.874$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 129$$

$$J = 0.432$$

$$226.951$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$
  

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$
  

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 70.83 \text{ in} \\ J &= 0.942 \\ &= 110.537 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 70.83 \\ J &= 0.942 \\ &= 110.537 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.63853$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.80939$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 10.5516 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 10.55 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 10.86 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-39.836	-39.836	0	0
2	M14	Y	-39.836	-39.836	0	0
3	M15	Y	-39.836	-39.836	0	0
4	M16	Y	-39.836	-39.836	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-40.786	-40.786	0	0
2	M14	y	-40.786	-40.786	0	0
3	M15	y	-65.613	-65.613	0	0
4	M16	y	-65.613	-65.613	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	92.212	92.212	0	0
2	M14	y	70.932	70.932	0	0
3	M15	y	39.013	39.013	0	0
4	M16	y	39.013	39.013	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 30° 90mph 30psf 10.75ft 7-05.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	130.033	1	959.983	3	210.596	1	.003	14	.391	1	1.455	1
20			min	7.296	12	-662.574	1	-127.675	14	-.012	2	.015	12	-2.063	3
21		11	max	130.033	1	545.419	1	-7.277	12	.012	2	.167	1	.734	1
22			min	7.296	12	-789.298	3	-165.457	1	0	3	.005	12	-1.018	3
23		12	max	130.033	1	428.263	1	-5.144	12	.012	2	.065	5	.152	1
24			min	7.296	12	-618.614	3	-120.318	1	0	3	-.004	1	-.177	3
25		13	max	130.033	1	311.107	1	-3.01	12	.012	2	.029	5	.46	3
26			min	7.296	12	-447.93	3	-75.18	1	0	3	-.121	1	-.289	1
27		14	max	130.033	1	193.952	1	-.876	12	.012	2	-.002	15	.893	3
28			min	7.296	12	-277.246	3	-32.818	4	0	3	-.183	1	-.591	1
29		15	max	130.033	1	76.796	1	15.098	1	.012	2	-.009	12	1.122	3
30			min	.553	15	-106.561	3	-22.149	5	0	3	-.192	1	-.753	1
31		16	max	130.033	1	64.123	3	60.236	1	.012	2	-.006	12	1.147	3
32			min	-11.129	5	-40.36	1	-18.848	5	0	3	-.147	1	-.774	1
33		17	max	130.033	1	234.807	3	105.375	1	.012	2	0	12	.969	3
34			min	-23.391	5	-157.516	1	-15.547	5	0	3	-.09	4	-.656	1
35		18	max	130.033	1	405.491	3	150.513	1	.012	2	.104	1	.586	3
36			min	-35.652	5	-274.671	1	-12.246	5	0	3	-.094	5	-.398	1
37		19	max	130.033	1	576.175	3	195.652	1	.012	2	.311	1	0	1
38			min	-47.913	5	-391.827	1	-8.945	5	0	3	-.107	5	0	3
39	M14	1	max	64.009	4	411.188	1	-10.051	12	.007	3	.352	1	0	4
40			min	3.048	12	-449.762	3	-201.367	1	-.009	2	.019	12	0	3
41		2	max	57.718	1	294.032	1	-7.917	12	.007	3	.191	4	.46	3
42			min	3.048	12	-319.747	3	-156.228	1	-.009	2	.009	12	-.421	1
43		3	max	57.718	1	176.876	1	-5.784	12	.007	3	.103	5	.764	3
44			min	3.048	12	-189.731	3	-111.089	1	-.009	2	-.021	1	-.702	1
45		4	max	57.718	1	59.721	1	-3.65	12	.007	3	.054	5	.913	3
46			min	3.048	12	-59.715	3	-65.951	1	-.009	2	-.127	1	-.844	1
47		5	max	57.718	1	70.301	3	-1.517	12	.007	3	.009	5	.906	3
48			min	.935	15	-57.435	1	-41.629	4	-.009	2	-.179	1	-.845	1
49		6	max	57.718	1	200.316	3	24.327	1	.007	3	-.009	12	.745	3
50			min	-10.798	5	-174.591	1	-32.735	5	-.009	2	-.177	1	-.707	1
51		7	max	57.718	1	330.332	3	69.465	1	.007	3	-.007	12	.428	3
52			min	-23.059	5	-291.746	1	-29.434	5	-.009	2	-.121	1	-.428	1
53		8	max	57.718	1	460.348	3	114.604	1	.007	3	0	10	0	9
54			min	-35.32	5	-408.902	1	-26.133	5	-.009	2	-.107	4	-.044	3
55		9	max	57.718	1	590.364	3	159.743	1	.007	3	.153	1	.549	1
56			min	-47.582	5	-526.058	1	-22.832	5	-.009	2	-.132	5	-.672	3
57		10	max	81.409	4	720.379	3	204.881	1	.007	3	.371	1	1.247	1
58			min	3.048	12	-643.213	1	-130.423	14	-.009	2	.015	12	-1.455	3
59		11	max	69.147	4	526.058	1	-7.018	12	.009	2	.192	4	.549	1
60			min	3.048	12	-590.364	3	-159.743	1	-.007	3	.005	12	-.672	3
61		12	max	57.718	1	408.902	1	-4.884	12	.009	2	.101	5	0	9
62			min	3.048	12	-460.348	3	-114.604	1	-.007	3	-.011	1	-.044	3
63		13	max	57.718	1	291.746	1	-2.75	12	.009	2	.052	5	.428	3
64			min	3.048	12	-330.332	3	-69.465	1	-.007	3	-.121	1	-.428	1
65		14	max	57.718	1	174.591	1	-.617	12	.009	2	.006	5	.745	3
66			min	3.048	12	-200.316	3	-42.507	4	-.007	3	-.177	1	-.707	1
67		15	max	57.718	1	57.435	1	20.812	1	.009	2	-.008	12	.906	3
68			min	3.048	12	-70.301	3	-32.945	5	-.007	3	-.179	1	-.845	1
69		16	max	57.718	1	59.715	3	65.951	1	.009	2	-.005	12	.913	3
70			min	-5.751	5	-59.721	1	-29.644	5	-.007	3	-.127	1	-.844	1
71		17	max	57.718	1	189.731	3	111.089	1	.009	2	.001	3	.764	3
72			min	-18.012	5	-176.876	1	-26.343	5	-.007	3	-.113	4	-.702	1
73		18	max	57.718	1	319.747	3	156.228	1	.009	2	.138	1	.46	3
74			min	-30.273	5	-294.032	1	-23.042	5	-.007	3	-.135	5	-.421	1
75		19	max	57.718	1	449.762	3	201.367	1	.009	2	.352	1	0	1



Company : Schletter, Inc.  
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Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76		min	-42.534	5	-411.188	1	-19.741	5	-.007	3	-.161	5	0	3
77	M15	max	90.061	5	550.35	2	-10.008	12	.009	2	.352	1	0	2
78		min	-60.856	1	-244.007	3	-201.333	1	-.006	3	.019	12	0	12
79		max	77.8	5	392.102	2	-7.875	12	.009	2	.23	4	.25	3
80		min	-60.856	1	-174.992	3	-156.194	1	-.006	3	.009	12	-.563	2
81		max	65.538	5	233.853	2	-5.741	12	.009	2	.13	5	.418	3
82		min	-60.856	1	-105.976	3	-111.056	1	-.006	3	-.021	1	-.937	2
83		max	53.277	5	75.604	2	-3.608	12	.009	2	.071	5	.503	3
84		min	-60.856	1	-36.961	3	-65.917	1	-.006	3	-.127	1	-1.121	2
85		max	41.016	5	32.054	3	-1.474	12	.009	2	.015	5	.506	3
86		min	-60.856	1	-82.645	2	-50.659	4	-.006	3	-.179	1	-1.117	2
87		max	28.755	5	101.069	3	24.36	1	.009	2	-.009	12	.427	3
88		min	-60.856	1	-240.894	2	-41.727	5	-.006	3	-.177	1	-.924	2
89		max	16.494	5	170.084	3	69.499	1	.009	2	-.007	12	.265	3
90		min	-60.856	1	-399.143	2	-38.426	5	-.006	3	-.121	1	-.542	2
91		max	4.232	5	239.099	3	114.637	1	.009	2	0	10	.029	2
92		min	-60.856	1	-557.392	2	-35.125	5	-.006	3	-.134	4	0	15
93		max	-3.487	12	308.114	3	159.776	1	.009	2	.153	1	.79	2
94		min	-60.856	1	-715.641	2	-31.824	5	-.006	3	-.169	5	-.306	3
95		max	-3.487	12	377.129	3	204.915	1	.009	2	.371	1	1.739	2
96		min	-60.856	1	-873.889	2	-135.041	14	-.006	3	.015	12	-.716	3
97		max	-.729	15	715.641	2	-7.06	12	.006	3	.229	4	.79	2
98		min	-60.856	1	-308.114	3	-159.776	1	-.009	2	.005	12	-.306	3
99		max	-3.487	12	557.392	2	-4.927	12	.006	3	.127	5	.029	2
100		min	-60.856	1	-239.099	3	-114.637	1	-.009	2	-.011	1	0	15
101		max	-3.487	12	399.143	2	-2.793	12	.006	3	.067	5	.265	3
102		min	-60.856	1	-170.084	3	-69.499	1	-.009	2	-.121	1	-.542	2
103		max	-3.487	12	240.894	2	-.659	12	.006	3	.011	5	.427	3
104		min	-60.856	1	-101.069	3	-51.567	4	-.009	2	-.177	1	-.924	2
105		max	-3.487	12	82.645	2	20.779	1	.006	3	-.008	12	.506	3
106		min	-65.421	4	-32.054	3	-41.942	5	-.009	2	-.179	1	-1.117	2
107		max	-3.487	12	36.961	3	65.917	1	.006	3	-.005	12	.503	3
108		min	-77.682	4	-75.604	2	-38.641	5	-.009	2	-.127	1	-1.121	2
109		max	-3.487	12	105.976	3	111.056	1	.006	3	0	3	.418	3
110		min	-89.943	4	-233.853	2	-35.34	5	-.009	2	-.14	4	-.937	2
111		max	-3.487	12	174.992	3	156.194	1	.006	3	.138	1	.25	3
112		min	-102.205	4	-392.102	2	-32.039	5	-.009	2	-.174	5	-.563	2
113		max	-3.487	12	244.007	3	201.333	1	.006	3	.352	1	0	2
114		min	-114.466	4	-550.35	2	-28.738	5	-.009	2	-.21	5	0	5
115	M16	max	87.999	5	530.923	2	-9.654	12	.009	1	.313	1	0	2
116		min	-139.477	1	-229.883	3	-195.888	1	-.009	3	.017	12	0	3
117		max	75.738	5	372.674	2	-7.521	12	.009	1	.177	4	.233	3
118		min	-139.477	1	-160.868	3	-150.75	1	-.009	3	.007	12	-.54	2
119		max	63.477	5	214.425	2	-5.387	12	.009	1	.1	5	.384	3
120		min	-139.477	1	-91.853	3	-105.611	1	-.009	3	-.048	1	-.89	2
121		max	51.215	5	56.176	2	-3.254	12	.009	1	.053	5	.453	3
122		min	-139.477	1	-22.838	3	-60.472	1	-.009	3	-.147	1	-1.052	2
123		max	38.954	5	46.177	3	-1.12	12	.009	1	.011	5	.439	3
124		min	-139.477	1	-102.073	2	-37.842	4	-.009	3	-.192	1	-1.024	2
125		max	26.693	5	115.192	3	29.805	1	.009	1	-.009	12	.342	3
126		min	-139.477	1	-260.321	2	-30.368	5	-.009	3	-.183	1	-.808	2
127		max	14.432	5	184.207	3	74.943	1	.009	1	-.006	12	.164	3
128		min	-139.477	1	-418.57	2	-27.067	5	-.009	3	-.121	1	-.403	2
129		max	2.171	5	253.222	3	120.082	1	.009	1	0	10	.192	2
130		min	-139.477	1	-576.819	2	-23.766	5	-.009	3	-.094	4	-.098	3
131		max	-6.588	15	322.237	3	165.221	1	.009	1	.166	1	.975	2
132		min	-139.477	1	-735.068	2	-20.465	5	-.009	3	-.118	5	-.441	3





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-7.547	12	391.252	3	210.359	1	.009	1	.39	1	1.948	2
134			min	-139.477	1	-893.317	2	-132.353	14	-.009	3	.016	12	-.867	3
135		11	max	-6.684	15	735.068	2	-7.414	12	.009	3	.183	4	.975	2
136			min	-139.477	1	-322.237	3	-165.221	1	-.009	1	.006	12	-.441	3
137		12	max	-7.547	12	576.819	2	-5.281	12	.009	3	.091	4	.192	2
138			min	-139.477	1	-253.222	3	-120.082	1	-.009	1	-.004	1	-.098	3
139		13	max	-7.547	12	418.57	2	-3.147	12	.009	3	.044	5	.164	3
140			min	-139.477	1	-184.207	3	-74.943	1	-.009	1	-.121	1	-.403	2
141		14	max	-7.547	12	260.321	2	-1.014	12	.009	3	0	15	.342	3
142			min	-139.477	1	-115.192	3	-42.125	4	-.009	1	-.183	1	-.808	2
143		15	max	-7.547	12	102.073	2	15.334	1	.009	3	-.009	12	.439	3
144			min	-139.477	1	-46.177	3	-31.395	5	-.009	1	-.192	1	-1.024	2
145		16	max	-7.547	12	22.838	3	60.472	1	.009	3	-.006	12	.453	3
146			min	-139.477	1	-56.176	2	-28.094	5	-.009	1	-.147	1	-1.052	2
147		17	max	-7.547	12	91.853	3	105.611	1	.009	3	-.001	12	.384	3
148			min	-139.477	1	-214.425	2	-24.793	5	-.009	1	-.119	4	-.89	2
149		18	max	-7.547	12	160.868	3	150.75	1	.009	3	.106	1	.233	3
150			min	-139.477	1	-372.674	2	-21.492	5	-.009	1	-.134	5	-.54	2
151		19	max	-7.547	12	229.883	3	195.888	1	.009	3	.313	1	0	2
152			min	-142.183	4	-530.923	2	-18.191	5	-.009	1	-.158	5	0	5
153	M2	1	max	899.693	1	1.957	4	.599	1	0	12	0	3	0	1
154			min	-1065.937	3	.473	15	-36.828	4	0	4	0	1	0	1
155		2	max	900.169	1	1.871	4	.599	1	0	12	0	1	0	15
156			min	-1065.58	3	.453	15	-37.244	4	0	4	-.012	4	0	4
157		3	max	900.645	1	1.786	4	.599	1	0	12	0	1	0	15
158			min	-1065.223	3	.433	15	-37.66	4	0	4	-.024	4	-.001	4
159		4	max	901.121	1	1.7	4	.599	1	0	12	0	1	0	15
160			min	-1064.866	3	.412	15	-38.077	4	0	4	-.036	4	-.002	4
161		5	max	901.596	1	1.615	4	.599	1	0	12	0	1	0	15
162			min	-1064.51	3	.392	15	-38.493	4	0	4	-.049	4	-.002	4
163		6	max	902.072	1	1.529	4	.599	1	0	12	0	1	0	15
164			min	-1064.153	3	.372	15	-38.909	4	0	4	-.061	4	-.003	4
165		7	max	902.548	1	1.443	4	.599	1	0	12	.001	1	0	15
166			min	-1063.796	3	.352	15	-39.326	4	0	4	-.074	4	-.003	4
167		8	max	903.024	1	1.358	4	.599	1	0	12	.001	1	0	15
168			min	-1063.439	3	.332	15	-39.742	4	0	4	-.087	4	-.004	4
169		9	max	903.499	1	1.272	4	.599	1	0	12	.002	1	-.001	15
170			min	-1063.082	3	.312	15	-40.159	4	0	4	-.1	4	-.004	4
171		10	max	903.975	1	1.187	4	.599	1	0	12	.002	1	-.001	15
172			min	-1062.725	3	.292	15	-40.575	4	0	4	-.113	4	-.005	4
173		11	max	904.451	1	1.101	4	.599	1	0	12	.002	1	-.001	15
174			min	-1062.369	3	.267	12	-40.991	4	0	4	-.126	4	-.005	4
175		12	max	904.927	1	1.015	4	.599	1	0	12	.002	1	-.001	15
176			min	-1062.012	3	.233	12	-41.408	4	0	4	-.139	4	-.005	4
177		13	max	905.402	1	.93	4	.599	1	0	12	.002	1	-.001	15
178			min	-1061.655	3	.2	12	-41.824	4	0	4	-.153	4	-.006	4
179		14	max	905.878	1	.844	4	.599	1	0	12	.003	1	-.001	15
180			min	-1061.298	3	.167	12	-42.24	4	0	4	-.166	4	-.006	4
181		15	max	906.354	1	.759	4	.599	1	0	12	.003	1	-.002	15
182			min	-1060.941	3	.133	12	-42.657	4	0	4	-.18	4	-.006	4
183		16	max	906.83	1	.679	2	.599	1	0	12	.003	1	-.002	15
184			min	-1060.585	3	.1	12	-43.073	4	0	4	-.194	4	-.006	4
185		17	max	907.305	1	.612	2	.599	1	0	12	.003	1	-.002	15
186			min	-1060.228	3	.067	12	-43.489	4	0	4	-.208	4	-.007	4
187		18	max	907.781	1	.546	2	.599	1	0	12	.003	1	-.002	15
188			min	-1059.871	3	.033	12	-43.906	4	0	4	-.222	4	-.007	4
189		19	max	908.257	1	.479	2	.599	1	0	12	.003	1	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1059.514	3	-.016	3	-44.322	4	0	4	-.237	4	-.007	4
191	M3	1	max	532.878	2	7.8	4	5.805	4	0	12	0	1	.007	4
192			min	-679.099	3	1.843	15	.015	12	0	4	-.034	4	.002	15
193		2	max	532.708	2	7.036	4	6.342	4	0	12	0	1	.004	2
194			min	-679.227	3	1.664	15	.015	12	0	4	-.032	4	0	12
195		3	max	532.537	2	6.271	4	6.879	4	0	12	0	1	.002	2
196			min	-679.355	3	1.484	15	.015	12	0	4	-.029	4	0	3
197		4	max	532.367	2	5.507	4	7.416	4	0	12	0	1	0	15
198			min	-679.483	3	1.304	15	.015	12	0	4	-.026	4	-.002	3
199		5	max	532.196	2	4.742	4	7.953	4	0	12	0	1	0	15
200			min	-679.61	3	1.125	15	.015	12	0	4	-.023	4	-.004	6
201		6	max	532.026	2	3.978	4	8.49	4	0	12	.001	1	-.001	15
202			min	-679.738	3	.945	15	.015	12	0	4	-.019	5	-.005	6
203		7	max	531.856	2	3.213	4	9.027	4	0	12	.001	1	-.002	15
204			min	-679.866	3	.765	15	.015	12	0	4	-.016	5	-.007	6
205		8	max	531.685	2	2.449	4	9.564	4	0	12	.001	1	-.002	15
206			min	-679.994	3	.586	15	.015	12	0	4	-.012	5	-.008	6
207		9	max	531.515	2	1.685	4	10.101	4	0	12	.001	1	-.002	15
208			min	-680.121	3	.406	15	.015	12	0	4	-.008	5	-.009	6
209		10	max	531.345	2	.92	4	10.638	4	0	12	.002	1	-.002	15
210			min	-680.249	3	.226	15	.015	12	0	4	-.004	5	-.009	6
211		11	max	531.174	2	.241	2	11.175	4	0	12	.002	1	-.002	15
212			min	-680.377	3	-.11	3	.015	12	0	4	0	12	-.01	6
213		12	max	531.004	2	-.133	15	11.712	4	0	12	.006	4	-.002	15
214			min	-680.505	3	-.61	6	.015	12	0	4	0	12	-.01	6
215		13	max	530.834	2	-.313	15	12.249	4	0	12	.011	4	-.002	15
216			min	-680.632	3	-1.374	6	.015	12	0	4	0	12	-.009	6
217		14	max	530.663	2	-.493	15	12.785	4	0	12	.016	4	-.002	15
218			min	-680.76	3	-2.138	6	.015	12	0	4	0	12	-.008	6
219		15	max	530.493	2	-.672	15	13.322	4	0	12	.022	4	-.002	15
220			min	-680.888	3	-2.903	6	.015	12	0	4	0	12	-.007	6
221		16	max	530.323	2	-.852	15	13.859	4	0	12	.027	4	-.001	15
222			min	-681.016	3	-3.667	6	.015	12	0	4	0	12	-.006	6
223		17	max	530.152	2	-1.032	15	14.396	4	0	12	.033	4	-.001	15
224			min	-681.144	3	-4.432	6	.015	12	0	4	0	12	-.004	6
225		18	max	529.982	2	-1.211	15	14.933	4	0	12	.039	4	0	15
226			min	-681.271	3	-5.196	6	.015	12	0	4	0	12	-.002	6
227		19	max	529.812	2	-1.391	15	15.47	4	0	12	.046	4	0	1
228			min	-681.399	3	-5.961	6	.015	12	0	4	0	12	0	1
229	M4	1	max	1046.9	1	0	1	-.738	12	0	1	.038	4	0	1
230			min	-30.56	5	0	1	-308.03	4	0	1	0	12	0	1
231		2	max	1047.071	1	0	1	-.738	12	0	1	.003	4	0	1
232			min	-30.481	5	0	1	-308.177	4	0	1	0	12	0	1
233		3	max	1047.241	1	0	1	-.738	12	0	1	0	12	0	1
234			min	-30.401	5	0	1	-308.325	4	0	1	-.033	4	0	1
235		4	max	1047.411	1	0	1	-.738	12	0	1	0	12	0	1
236			min	-30.322	5	0	1	-308.473	4	0	1	-.068	4	0	1
237		5	max	1047.582	1	0	1	-.738	12	0	1	0	12	0	1
238			min	-30.242	5	0	1	-308.62	4	0	1	-.104	4	0	1
239		6	max	1047.752	1	0	1	-.738	12	0	1	0	12	0	1
240			min	-30.163	5	0	1	-308.768	4	0	1	-.139	4	0	1
241		7	max	1047.922	1	0	1	-.738	12	0	1	0	12	0	1
242			min	-30.083	5	0	1	-308.916	4	0	1	-.174	4	0	1
243		8	max	1048.093	1	0	1	-.738	12	0	1	0	12	0	1
244			min	-30.004	5	0	1	-309.063	4	0	1	-.21	4	0	1
245		9	max	1048.263	1	0	1	-.738	12	0	1	0	12	0	1
246			min	-29.924	5	0	1	-309.211	4	0	1	-.245	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1048.433	1	0	1	-.738	12	0	1	0	12	0	1
248		min	-29.845	5	0	1	-309.358	4	0	1	-.281	4	0	1
249	11	max	1048.604	1	0	1	-.738	12	0	1	0	12	0	1
250		min	-29.765	5	0	1	-309.506	4	0	1	-.317	4	0	1
251	12	max	1048.774	1	0	1	-.738	12	0	1	0	12	0	1
252		min	-29.686	5	0	1	-309.654	4	0	1	-.352	4	0	1
253	13	max	1048.944	1	0	1	-.738	12	0	1	0	12	0	1
254		min	-29.606	5	0	1	-309.801	4	0	1	-.388	4	0	1
255	14	max	1049.115	1	0	1	-.738	12	0	1	0	12	0	1
256		min	-29.527	5	0	1	-309.949	4	0	1	-.423	4	0	1
257	15	max	1049.285	1	0	1	-.738	12	0	1	-.001	12	0	1
258		min	-29.447	5	0	1	-310.097	4	0	1	-.459	4	0	1
259	16	max	1049.455	1	0	1	-.738	12	0	1	-.001	12	0	1
260		min	-29.368	5	0	1	-310.244	4	0	1	-.494	4	0	1
261	17	max	1049.626	1	0	1	-.738	12	0	1	-.001	12	0	1
262		min	-29.288	5	0	1	-310.392	4	0	1	-.53	4	0	1
263	18	max	1049.796	1	0	1	-.738	12	0	1	-.001	12	0	1
264		min	-29.209	5	0	1	-310.539	4	0	1	-.566	4	0	1
265	19	max	1049.967	1	0	1	-.738	12	0	1	-.001	12	0	1
266		min	-29.129	5	0	1	-310.687	4	0	1	-.601	4	0	1
267	M6	1	max	2904.857	1	2.149	2	0	1	0	0	4	0	1
268		min	-3501.162	3	.263	12	-37.227	4	0	4	0	1	0	1
269	2	max	2905.333	1	2.082	2	0	1	0	1	0	1	0	12
270		min	-3500.805	3	.23	12	-37.643	4	0	4	-.012	4	0	2
271	3	max	2905.809	1	2.016	2	0	1	0	1	0	1	0	12
272		min	-3500.448	3	.196	12	-38.06	4	0	4	-.024	4	-.001	2
273	4	max	2906.284	1	1.949	2	0	1	0	1	0	1	0	12
274		min	-3500.092	3	.163	12	-38.476	4	0	4	-.037	4	-.002	2
275	5	max	2906.76	1	1.882	2	0	1	0	1	0	1	0	12
276		min	-3499.735	3	.129	12	-38.893	4	0	4	-.049	4	-.003	2
277	6	max	2907.236	1	1.816	2	0	1	0	1	0	1	0	12
278		min	-3499.378	3	.095	3	-39.309	4	0	4	-.062	4	-.003	2
279	7	max	2907.712	1	1.749	2	0	1	0	1	0	1	0	12
280		min	-3499.021	3	.045	3	-39.725	4	0	4	-.075	4	-.004	2
281	8	max	2908.187	1	1.682	2	0	1	0	1	0	1	0	12
282		min	-3498.664	3	-.005	3	-40.142	4	0	4	-.088	4	-.004	2
283	9	max	2908.663	1	1.615	2	0	1	0	1	0	1	0	12
284		min	-3498.308	3	-.055	3	-40.558	4	0	4	-.101	4	-.005	2
285	10	max	2909.139	1	1.549	2	0	1	0	1	0	1	0	12
286		min	-3497.951	3	-.105	3	-40.974	4	0	4	-.114	4	-.005	2
287	11	max	2909.615	1	1.482	2	0	1	0	1	0	1	0	3
288		min	-3497.594	3	-.155	3	-41.391	4	0	4	-.127	4	-.006	2
289	12	max	2910.09	1	1.415	2	0	1	0	1	0	1	0	3
290		min	-3497.237	3	-.205	3	-41.807	4	0	4	-.141	4	-.006	2
291	13	max	2910.566	1	1.349	2	0	1	0	1	0	1	0	3
292		min	-3496.88	3	-.255	3	-42.223	4	0	4	-.154	4	-.007	2
293	14	max	2911.042	1	1.282	2	0	1	0	1	0	1	0	3
294		min	-3496.523	3	-.305	3	-42.64	4	0	4	-.168	4	-.007	2
295	15	max	2911.518	1	1.215	2	0	1	0	1	0	1	0	3
296		min	-3496.167	3	-.355	3	-43.056	4	0	4	-.182	4	-.008	2
297	16	max	2911.993	1	1.149	2	0	1	0	1	0	1	0	3
298		min	-3495.81	3	-.405	3	-43.472	4	0	4	-.196	4	-.008	2
299	17	max	2912.469	1	1.082	2	0	1	0	1	0	1	0	3
300		min	-3495.453	3	-.455	3	-43.889	4	0	4	-.21	4	-.008	2
301	18	max	2912.945	1	1.015	2	0	1	0	1	0	1	0	3
302		min	-3495.096	3	-.505	3	-44.305	4	0	4	-.224	4	-.009	2
303	19	max	2913.421	1	.949	2	0	1	0	1	0	1	0	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-3494.739	3	-.555	3	-44.721	4	0	4	-.239	4	-.009	2
305	M7	1	max	2084.589	2	7.813	6	5.481	4	0	1	0	1	.009	2
306			min	-2139.19	3	1.834	15	0	1	0	4	-.034	4	0	3
307		2	max	2084.418	2	7.049	6	6.018	4	0	1	0	1	.006	2
308			min	-2139.318	3	1.654	15	0	1	0	4	-.032	4	-.002	3
309		3	max	2084.248	2	6.284	6	6.555	4	0	1	0	1	.004	2
310			min	-2139.446	3	1.475	15	0	1	0	4	-.029	4	-.004	3
311		4	max	2084.078	2	5.52	6	7.092	4	0	1	0	1	.002	2
312			min	-2139.574	3	1.295	15	0	1	0	4	-.027	4	-.005	3
313		5	max	2083.907	2	4.755	6	7.629	4	0	1	0	1	0	2
314			min	-2139.702	3	1.115	15	0	1	0	4	-.024	4	-.006	3
315		6	max	2083.737	2	3.991	6	8.166	4	0	1	0	1	-.001	15
316			min	-2139.829	3	.936	15	0	1	0	4	-.02	4	-.007	3
317		7	max	2083.567	2	3.226	6	8.703	4	0	1	0	1	-.002	15
318			min	-2139.957	3	.756	15	0	1	0	4	-.017	4	-.007	3
319		8	max	2083.396	2	2.462	6	9.24	4	0	1	0	1	-.002	15
320			min	-2140.085	3	.567	12	0	1	0	4	-.013	4	-.008	4
321		9	max	2083.226	2	1.797	2	9.777	4	0	1	0	1	-.002	15
322			min	-2140.213	3	.269	12	0	1	0	4	-.009	4	-.009	4
323		10	max	2083.055	2	1.201	2	10.314	4	0	1	0	1	-.002	15
324			min	-2140.34	3	-.082	3	0	1	0	4	-.005	4	-.009	4
325		11	max	2082.885	2	.605	2	10.851	4	0	1	0	1	-.002	15
326			min	-2140.468	3	-.528	3	0	1	0	4	0	5	-.01	4
327		12	max	2082.715	2	.01	2	11.388	4	0	1	.004	4	-.002	15
328			min	-2140.596	3	-.975	3	0	1	0	4	0	1	-.01	4
329		13	max	2082.544	2	-.322	15	11.924	4	0	1	.009	4	-.002	15
330			min	-2140.724	3	-1.422	3	0	1	0	4	0	1	-.009	4
331		14	max	2082.374	2	-.502	15	12.461	4	0	1	.014	4	-.002	15
332			min	-2140.851	3	-2.125	4	0	1	0	4	0	1	-.008	4
333		15	max	2082.204	2	-.682	15	12.998	4	0	1	.02	4	-.002	15
334			min	-2140.979	3	-2.889	4	0	1	0	4	0	1	-.007	4
335		16	max	2082.033	2	-.861	15	13.535	4	0	1	.025	4	-.001	15
336			min	-2141.107	3	-3.654	4	0	1	0	4	0	1	-.006	4
337		17	max	2081.863	2	-1.041	15	14.072	4	0	1	.031	4	-.001	15
338			min	-2141.235	3	-4.418	4	0	1	0	4	0	1	-.004	4
339		18	max	2081.693	2	-1.221	15	14.609	4	0	1	.037	4	0	15
340			min	-2141.362	3	-5.182	4	0	1	0	4	0	1	-.002	4
341		19	max	2081.522	2	-1.4	15	15.146	4	0	1	.043	4	0	1
342			min	-2141.49	3	-5.947	4	0	1	0	4	0	1	0	1
343	M8	1	max	2673.792	1	0	1	0	1	0	1	.036	4	0	1
344			min	-106.548	3	0	1	-297.034	4	0	1	0	1	0	1
345		2	max	2673.963	1	0	1	0	1	0	1	.002	5	0	1
346			min	-106.42	3	0	1	-297.182	4	0	1	0	1	0	1
347		3	max	2674.133	1	0	1	0	1	0	1	0	1	0	1
348			min	-106.292	3	0	1	-297.329	4	0	1	-.033	4	0	1
349		4	max	2674.303	1	0	1	0	1	0	1	0	1	0	1
350			min	-106.164	3	0	1	-297.477	4	0	1	-.067	4	0	1
351		5	max	2674.474	1	0	1	0	1	0	1	0	1	0	1
352			min	-106.037	3	0	1	-297.625	4	0	1	-.101	4	0	1
353		6	max	2674.644	1	0	1	0	1	0	1	0	1	0	1
354			min	-105.909	3	0	1	-297.772	4	0	1	-.135	4	0	1
355		7	max	2674.814	1	0	1	0	1	0	1	0	1	0	1
356			min	-105.781	3	0	1	-297.92	4	0	1	-.169	4	0	1
357		8	max	2674.985	1	0	1	0	1	0	1	0	1	0	1
358			min	-105.653	3	0	1	-298.068	4	0	1	-.203	4	0	1
359		9	max	2675.155	1	0	1	0	1	0	1	0	1	0	1
360			min	-105.526	3	0	1	-298.215	4	0	1	-.238	4	0	1



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2675.325	1	0	1	0	1	0	1	0	1	0	1
362			min	-105.398	3	0	1	-298.363	4	0	1	-.272	4	0	1
363		11	max	2675.496	1	0	1	0	1	0	1	0	1	0	1
364			min	-105.27	3	0	1	-298.51	4	0	1	-.306	4	0	1
365		12	max	2675.666	1	0	1	0	1	0	1	0	1	0	1
366			min	-105.142	3	0	1	-298.658	4	0	1	-.34	4	0	1
367		13	max	2675.836	1	0	1	0	1	0	1	0	1	0	1
368			min	-105.015	3	0	1	-298.806	4	0	1	-.375	4	0	1
369		14	max	2676.007	1	0	1	0	1	0	1	0	1	0	1
370			min	-104.887	3	0	1	-298.953	4	0	1	-.409	4	0	1
371		15	max	2676.177	1	0	1	0	1	0	1	0	1	0	1
372			min	-104.759	3	0	1	-299.101	4	0	1	-.443	4	0	1
373		16	max	2676.348	1	0	1	0	1	0	1	0	1	0	1
374			min	-104.631	3	0	1	-299.249	4	0	1	-.478	4	0	1
375		17	max	2676.518	1	0	1	0	1	0	1	0	1	0	1
376			min	-104.504	3	0	1	-299.396	4	0	1	-.512	4	0	1
377		18	max	2676.688	1	0	1	0	1	0	1	0	1	0	1
378			min	-104.376	3	0	1	-299.544	4	0	1	-.547	4	0	1
379		19	max	2676.859	1	0	1	0	1	0	1	0	1	0	1
380			min	-104.248	3	0	1	-299.692	4	0	1	-.581	4	0	1
381	M10	1	max	899.693	1	1.9	6	-.03	12	0	1	0	4	0	1
382			min	-1065.937	3	.435	15	-37.196	4	0	5	0	3	0	1
383		2	max	900.169	1	1.815	6	-.03	12	0	1	0	10	0	15
384			min	-1065.58	3	.415	15	-37.613	4	0	5	-.012	4	0	6
385		3	max	900.645	1	1.729	6	-.03	12	0	1	0	12	0	15
386			min	-1065.223	3	.395	15	-38.029	4	0	5	-.024	4	-.001	6
387		4	max	901.121	1	1.644	6	-.03	12	0	1	0	12	0	15
388			min	-1064.866	3	.374	15	-38.445	4	0	5	-.037	4	-.002	6
389		5	max	901.596	1	1.558	6	-.03	12	0	1	0	12	0	15
390			min	-1064.51	3	.354	15	-38.862	4	0	5	-.049	4	-.002	6
391		6	max	902.072	1	1.473	6	-.03	12	0	1	0	12	0	15
392			min	-1064.153	3	.334	15	-39.278	4	0	5	-.062	4	-.003	6
393		7	max	902.548	1	1.387	6	-.03	12	0	1	0	12	0	15
394			min	-1063.796	3	.314	15	-39.694	4	0	5	-.075	4	-.003	6
395		8	max	903.024	1	1.301	6	-.03	12	0	1	0	12	0	15
396			min	-1063.439	3	.294	15	-40.111	4	0	5	-.088	4	-.004	6
397		9	max	903.499	1	1.216	6	-.03	12	0	1	0	12	0	15
398			min	-1063.082	3	.274	15	-40.527	4	0	5	-.101	4	-.004	6
399		10	max	903.975	1	1.13	6	-.03	12	0	1	0	12	-.001	15
400			min	-1062.725	3	.254	15	-40.944	4	0	5	-.114	4	-.004	6
401		11	max	904.451	1	1.045	6	-.03	12	0	1	0	12	-.001	15
402			min	-1062.369	3	.234	15	-41.36	4	0	5	-.127	4	-.005	6
403		12	max	904.927	1	.959	6	-.03	12	0	1	0	12	-.001	15
404			min	-1062.012	3	.214	15	-41.776	4	0	5	-.141	4	-.005	6
405		13	max	905.402	1	.879	2	-.03	12	0	1	0	12	-.001	15
406			min	-1061.655	3	.193	15	-42.193	4	0	5	-.154	4	-.005	6
407		14	max	905.878	1	.813	2	-.03	12	0	1	0	12	-.001	15
408			min	-1061.298	3	.167	12	-42.609	4	0	5	-.168	4	-.006	6
409		15	max	906.354	1	.746	2	-.03	12	0	1	0	12	-.001	15
410			min	-1060.941	3	.133	12	-43.025	4	0	5	-.182	4	-.006	6
411		16	max	906.83	1	.679	2	-.03	12	0	1	0	12	-.001	15
412			min	-1060.585	3	.1	12	-43.442	4	0	5	-.196	4	-.006	6
413		17	max	907.305	1	.612	2	-.03	12	0	1	0	12	-.001	15
414			min	-1060.228	3	.067	12	-43.858	4	0	5	-.21	4	-.006	6
415		18	max	907.781	1	.546	2	-.03	12	0	1	0	12	-.001	15
416			min	-1059.871	3	.033	12	-44.274	4	0	5	-.224	4	-.006	6
417		19	max	908.257	1	.479	2	-.03	12	0	1	0	12	-.001	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1059.514	3	-.016	3	-44.691	4	0	5	-.239	4	-.007	6
419	M11	1	max	532.878	2	7.756	6	5.634	4	0	1	0	12	.007	6
420			min	-679.099	3	1.814	15	-.279	1	0	4	-.034	4	.001	15
421		2	max	532.708	2	6.992	6	6.171	4	0	1	0	12	.004	2
422			min	-679.227	3	1.634	15	-.279	1	0	4	-.032	4	0	12
423		3	max	532.537	2	6.227	6	6.708	4	0	1	0	12	.002	2
424			min	-679.355	3	1.455	15	-.279	1	0	4	-.029	4	0	3
425		4	max	532.367	2	5.463	6	7.245	4	0	1	0	12	0	2
426			min	-679.483	3	1.275	15	-.279	1	0	4	-.026	4	-.002	3
427		5	max	532.196	2	4.699	6	7.782	4	0	1	0	12	0	15
428			min	-679.61	3	1.095	15	-.279	1	0	4	-.023	4	-.004	4
429		6	max	532.026	2	3.934	6	8.319	4	0	1	0	12	-.001	15
430			min	-679.738	3	.916	15	-.279	1	0	4	-.02	4	-.006	4
431		7	max	531.856	2	3.17	6	8.855	4	0	1	0	12	-.002	15
432			min	-679.866	3	.736	15	-.279	1	0	4	-.016	4	-.007	4
433		8	max	531.685	2	2.405	6	9.392	4	0	1	0	12	-.002	15
434			min	-679.994	3	.556	15	-.279	1	0	4	-.012	4	-.008	4
435		9	max	531.515	2	1.641	6	9.929	4	0	1	0	12	-.002	15
436			min	-680.121	3	.376	15	-.279	1	0	4	-.008	4	-.009	4
437		10	max	531.345	2	.876	6	10.466	4	0	1	0	12	-.002	15
438			min	-680.249	3	.197	15	-.279	1	0	4	-.004	4	-.01	4
439		11	max	531.174	2	.241	2	11.003	4	0	1	0	5	-.002	15
440			min	-680.377	3	-.11	3	-.279	1	0	4	-.002	1	-.01	4
441		12	max	531.004	2	-.163	15	11.54	4	0	1	.005	5	-.002	15
442			min	-680.505	3	-.653	4	-.279	1	0	4	-.002	1	-.01	4
443		13	max	530.834	2	-.342	15	12.077	4	0	1	.01	5	-.002	15
444			min	-680.632	3	-1.418	4	-.279	1	0	4	-.002	1	-.009	4
445		14	max	530.663	2	-.522	15	12.614	4	0	1	.015	5	-.002	15
446			min	-680.76	3	-2.182	4	-.279	1	0	4	-.002	1	-.009	4
447		15	max	530.493	2	-.702	15	13.151	4	0	1	.021	5	-.002	15
448			min	-680.888	3	-2.947	4	-.279	1	0	4	-.002	1	-.007	4
449		16	max	530.323	2	-.881	15	13.688	4	0	1	.026	5	-.001	15
450			min	-681.016	3	-3.711	4	-.279	1	0	4	-.002	1	-.006	4
451		17	max	530.152	2	-1.061	15	14.225	4	0	1	.032	5	-.001	15
452			min	-681.144	3	-4.476	4	-.279	1	0	4	-.002	1	-.004	4
453		18	max	529.982	2	-1.241	15	14.762	4	0	1	.038	5	0	15
454			min	-681.271	3	-5.24	4	-.279	1	0	4	-.002	1	-.002	4
455		19	max	529.812	2	-1.42	15	15.299	4	0	1	.044	5	0	1
456			min	-681.399	3	-6.004	4	-.279	1	0	4	-.003	1	0	1
457	M12	1	max	1046.9	1	0	1	13.741	1	0	1	.037	5	0	1
458			min	-3.225	3	0	1	-299.553	4	0	1	-.002	1	0	1
459		2	max	1047.071	1	0	1	13.741	1	0	1	.003	5	0	1
460			min	-3.097	3	0	1	-299.7	4	0	1	0	1	0	1
461		3	max	1047.241	1	0	1	13.741	1	0	1	.001	1	0	1
462			min	-2.97	3	0	1	-299.848	4	0	1	-.032	4	0	1
463		4	max	1047.411	1	0	1	13.741	1	0	1	.003	1	0	1
464			min	-2.842	3	0	1	-299.996	4	0	1	-.067	4	0	1
465		5	max	1047.582	1	0	1	13.741	1	0	1	.004	1	0	1
466			min	-2.714	3	0	1	-300.143	4	0	1	-.101	4	0	1
467		6	max	1047.752	1	0	1	13.741	1	0	1	.006	1	0	1
468			min	-2.586	3	0	1	-300.291	4	0	1	-.135	4	0	1
469		7	max	1047.922	1	0	1	13.741	1	0	1	.007	1	0	1
470			min	-2.459	3	0	1	-300.439	4	0	1	-.17	4	0	1
471		8	max	1048.093	1	0	1	13.741	1	0	1	.009	1	0	1
472			min	-2.331	3	0	1	-300.586	4	0	1	-.204	4	0	1
473		9	max	1048.263	1	0	1	13.741	1	0	1	.01	1	0	1
474			min	-2.203	3	0	1	-300.734	4	0	1	-.239	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1048.433	1	0	1	13.741	1	0	1	.012	1	0	1
476			min	-2.075	3	0	1	-300.881	4	0	1	-.274	4	0	1
477		11	max	1048.604	1	0	1	13.741	1	0	1	.014	1	0	1
478			min	-1.947	3	0	1	-301.029	4	0	1	-.308	4	0	1
479		12	max	1048.774	1	0	1	13.741	1	0	1	.015	1	0	1
480			min	-1.82	3	0	1	-301.177	4	0	1	-.343	4	0	1
481		13	max	1048.944	1	0	1	13.741	1	0	1	.017	1	0	1
482			min	-1.692	3	0	1	-301.324	4	0	1	-.377	4	0	1
483		14	max	1049.115	1	0	1	13.741	1	0	1	.018	1	0	1
484			min	-1.564	3	0	1	-301.472	4	0	1	-.412	4	0	1
485		15	max	1049.285	1	0	1	13.741	1	0	1	.02	1	0	1
486			min	-1.436	3	0	1	-301.62	4	0	1	-.447	4	0	1
487		16	max	1049.455	1	0	1	13.741	1	0	1	.022	1	0	1
488			min	-1.309	3	0	1	-301.767	4	0	1	-.481	4	0	1
489		17	max	1049.626	1	0	1	13.741	1	0	1	.023	1	0	1
490			min	-1.181	3	0	1	-301.915	4	0	1	-.516	4	0	1
491		18	max	1049.796	1	0	1	13.741	1	0	1	.025	1	0	1
492			min	-1.053	3	0	1	-302.063	4	0	1	-.55	4	0	1
493		19	max	1049.967	1	0	1	13.741	1	0	1	.026	1	0	1
494			min	-.925	3	0	1	-302.21	4	0	1	-.585	4	0	1
495	M1	1	max	195.658	1	576.148	3	47.881	5	0	1	.311	1	0	3
496			min	-8.945	5	-390.504	1	-129.855	1	0	3	-.107	5	-.012	2
497		2	max	196.375	1	575.218	3	49.122	5	0	1	.243	1	.196	1
498			min	-8.611	5	-391.744	1	-129.855	1	0	3	-.081	5	-.303	3
499		3	max	416.136	3	439.107	1	14.484	5	0	3	.174	1	.393	1
500			min	-243.038	2	-415.63	3	-129.521	1	0	1	-.055	5	-.594	3
501		4	max	416.673	3	437.866	1	15.725	5	0	3	.106	1	.161	1
502			min	-242.322	2	-416.561	3	-129.521	1	0	1	-.047	5	-.375	3
503		5	max	417.21	3	436.626	1	16.966	5	0	3	.037	1	-.003	15
504			min	-241.606	2	-417.491	3	-129.521	1	0	1	-.039	5	-.155	3
505		6	max	417.747	3	435.385	1	18.208	5	0	3	-.002	12	.066	3
506			min	-240.89	2	-418.422	3	-129.521	1	0	1	-.037	4	-.317	2
507		7	max	418.284	3	434.144	1	19.449	5	0	3	-.006	12	.287	3
508			min	-240.173	2	-419.352	3	-129.521	1	0	1	-.099	1	-.545	2
509		8	max	418.822	3	432.904	1	20.691	5	0	3	-.006	15	.508	3
510			min	-239.457	2	-420.282	3	-129.521	1	0	1	-.168	1	-.772	2
511		9	max	434.486	3	40.588	2	64.348	5	0	9	.098	1	.594	3
512			min	-154.631	2	.375	15	-187.278	1	0	3	-.145	5	-.885	2
513		10	max	435.023	3	39.347	2	65.589	5	0	9	0	12	.578	3
514			min	-153.915	2	0	15	-187.278	1	0	3	-.112	4	-.906	2
515		11	max	435.56	3	38.107	2	66.831	5	0	9	-.006	12	.563	3
516			min	-153.199	2	-1.529	4	-187.278	1	0	3	-.1	1	-.926	2
517		12	max	451.158	3	274.156	3	167.326	5	0	2	.166	1	.49	3
518			min	-92.161	10	-513.026	2	-126.506	1	0	3	-.229	5	-.821	2
519		13	max	451.695	3	273.226	3	168.568	5	0	2	.099	1	.346	3
520			min	-91.564	10	-514.267	2	-126.506	1	0	3	-.141	5	-.55	2
521		14	max	452.232	3	272.295	3	169.809	5	0	2	.032	1	.202	3
522			min	-90.967	10	-515.507	2	-126.506	1	0	3	-.051	5	-.28	1
523		15	max	452.769	3	271.365	3	171.051	5	0	2	.039	5	.058	3
524			min	-90.37	10	-516.748	2	-126.506	1	0	3	-.035	1	-.027	1
525		16	max	453.306	3	270.434	3	172.292	5	0	2	.129	5	.267	2
526			min	-89.774	10	-517.988	2	-126.506	1	0	3	-.101	1	-.085	3
527		17	max	453.843	3	269.504	3	173.533	5	0	2	.221	5	.541	2
528			min	-89.177	10	-519.229	2	-126.506	1	0	3	-.168	1	-.227	3
529		18	max	17.856	5	532.673	2	-7.548	12	0	5	.215	5	.272	2
530			min	-196.599	1	-229.017	3	-143.583	4	0	2	-.239	1	-.113	3
531		19	max	18.19	5	531.433	2	-7.548	12	0	5	.158	5	.009	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC	
532	M5	1	min	-195.883	1	-229.948	3	-142.342	4	0	2	-.313	1	-.009	1	
533		1	max	421.179	1	1919.856	3	106.052	5	0	1	0	1	.023	2	
534			min	18.823	12	-1317.137	1	0	1	0	4	-.246	4	-.002	3	
535		2	max	421.895	1	1918.926	3	107.293	5	0	1	0	1	.717	1	
536			min	19.181	12	-1318.378	1	0	1	0	4	-.19	4	-1.014	3	
537		3	max	1340.638	3	1351.062	1	76.225	4	0	4	0	1	1.38	1	
538			min	-876.175	2	-1347.708	3	0	1	0	1	-.134	4	-1.987	3	
539		4	max	1341.175	3	1349.821	1	77.467	4	0	4	0	1	.668	1	
540			min	-875.459	2	-1348.638	3	0	1	0	1	-.093	4	-1.276	3	
541		5	max	1341.713	3	1348.581	1	78.708	4	0	4	0	1	.003	9	
542		min	-874.742	2	-1349.569	3	0	1	0	1	-.052	4	-.564	3		
543		6	max	1342.25	3	1347.34	1	79.95	4	0	4	0	1	.148	3	
544			min	-874.026	2	-1350.499	3	0	1	0	1	-.01	5	-.799	2	
545		7	max	1342.787	3	1346.1	1	81.191	4	0	4	.033	4	.861	3	
546			min	-873.31	2	-1351.43	3	0	1	0	1	0	1	-1.502	2	
547		8	max	1343.324	3	1344.859	1	82.432	4	0	4	.076	4	1.574	3	
548			min	-872.594	2	-1352.36	3	0	1	0	1	0	1	-2.204	2	
549		9	max	1371.213	3	135.038	2	212.047	4	0	1	0	1	1.812	3	
550			min	-698.657	2	.376	15	0	1	0	1	-.215	4	-2.51	2	
551		10	max	1371.751	3	133.797	2	213.289	4	0	1	0	1	1.756	3	
552			min	-697.94	2	.002	15	0	1	0	1	-.103	4	-2.581	2	
553		11	max	1372.288	3	132.557	2	214.53	4	0	1	.01	4	1.701	3	
554			min	-697.224	2	-1.334	6	0	1	0	1	0	1	-2.651	2	
555		12	max	1400.311	3	880.272	3	246.582	4	0	1	0	1	1.494	3	
556			min	-523.307	2	-1603.986	2	0	1	0	4	-.341	4	-2.374	2	
557		13	max	1400.848	3	879.341	3	247.824	4	0	1	0	1	1.03	3	
558			min	-522.591	2	-1605.226	2	0	1	0	4	-.21	4	-1.527	2	
559		14	max	1401.385	3	878.411	3	249.065	4	0	1	0	1	.566	3	
560			min	-521.875	2	-1606.467	2	0	1	0	4	-.079	4	-.693	1	
561		15	max	1401.922	3	877.481	3	250.307	4	0	1	.053	4	.168	2	
562			min	-521.159	2	-1607.708	2	0	1	0	4	0	1	-.004	13	
563		16	max	1402.459	3	876.55	3	251.548	4	0	1	.185	4	1.017	2	
564			min	-520.442	2	-1608.948	2	0	1	0	4	0	1	-.36	3	
565		17	max	1402.996	3	875.62	3	252.79	4	0	1	.318	4	1.866	2	
566			min	-519.726	2	-1610.189	2	0	1	0	4	0	1	-.822	3	
567		18	max	-19.453	12	1790.864	2	0	1	0	4	.356	4	.962	2	
568			min	-421.445	1	-781.988	3	-25.572	5	0	1	0	1	-.43	3	
569		19	max	-19.095	12	1789.624	2	0	1	0	4	.344	4	.019	1	
570			min	-420.729	1	-782.918	3	-24.331	5	0	1	0	1	-.017	3	
571		M9	1	max	195.658	1	576.148	3	129.855	1	0	3	-.018	12	0	3
572			min	9.791	12	-390.504	1	7.296	12	0	4	-.311	1	-.012	2	
573		2	max	196.375	1	575.218	3	129.855	1	0	3	-.014	12	.196	1	
574			min	10.149	12	-391.744	1	7.296	12	0	4	-.243	1	-.303	3	
575		3	max	416.136	3	439.107	1	129.521	1	0	1	-.01	12	.393	1	
576			min	-243.038	2	-415.63	3	7.265	12	0	3	-.174	1	-.594	3	
577		4	max	416.673	3	437.866	1	129.521	1	0	1	-.006	12	.161	1	
578			min	-242.322	2	-416.561	3	7.265	12	0	3	-.106	1	-.375	3	
579		5	max	417.21	3	436.626	1	129.521	1	0	1	-.002	12	-.003	15	
580			min	-241.606	2	-417.491	3	7.265	12	0	3	-.053	4	-.155	3	
581		6	max	417.747	3	435.385	1	129.521	1	0	1	.031	1	.066	3	
582			min	-240.89	2	-418.422	3	7.265	12	0	3	-.025	5	-.317	2	
583		7	max	418.284	3	434.144	1	129.521	1	0	1	.099	1	.287	3	
584			min	-240.173	2	-419.352	3	7.265	12	0	3	-.005	5	-.545	2	
585		8	max	418.822	3	432.904	1	129.521	1	0	1	.168	1	.508	3	
586			min	-239.457	2	-420.282	3	7.265	12	0	3	.009	12	-.772	2	
587		9	max	434.486	3	40.588	2	187.278	1	0	3	-.005	12	.594	3	
588			min	-154.631	2	.382	15	10.308	12	0	9	-.184	4	-.885	2	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	435.023	3	39.347	2	187.278	1	0	3	.001	1	.578	3
590		min	-153.915	2	.008	15	10.308	12	0	9	-.111	4	-.906	2
591	11	max	435.56	3	38.107	2	187.278	1	0	3	.1	1	.563	3
592		min	-153.199	2	-1.478	6	10.308	12	0	9	-.061	5	-.926	2
593	12	max	451.158	3	274.156	3	218.184	4	0	3	-.009	12	.49	3
594		min	-92.161	10	-513.026	2	6.826	12	0	2	-.295	4	-.821	2
595	13	max	451.695	3	273.226	3	219.426	4	0	3	-.005	12	.346	3
596		min	-91.564	10	-514.267	2	6.826	12	0	2	-.18	4	-.55	2
597	14	max	452.232	3	272.295	3	220.667	4	0	3	-.002	12	.202	3
598		min	-90.967	10	-515.507	2	6.826	12	0	2	-.064	4	-.28	1
599	15	max	452.769	3	271.365	3	221.909	4	0	3	.053	4	.058	3
600		min	-90.37	10	-516.748	2	6.826	12	0	2	.002	12	-.027	1
601	16	max	453.306	3	270.434	3	223.15	4	0	3	.17	4	.267	2
602		min	-89.774	10	-517.988	2	6.826	12	0	2	.005	12	-.085	3
603	17	max	453.843	3	269.504	3	224.392	4	0	3	.288	4	.541	2
604		min	-89.177	10	-519.229	2	6.826	12	0	2	.009	12	-.227	3
605	18	max	-10.013	12	532.673	2	139.649	1	0	2	.31	4	.272	2
606		min	-196.599	1	-229.017	3	-89.488	5	0	3	.013	12	-.113	3
607	19	max	-9.655	12	531.433	2	139.649	1	0	2	.313	1	.009	3
608		min	-195.883	1	-229.948	3	-88.246	5	0	3	.017	12	-.009	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.09	2	.007	3	7.536e-3	2	NC	1	NC	1
2			min	-.761	4	-.011	3	-.003	2	-1.226e-3	3	NC	1	NC	1
3		2	max	.001	1	.327	3	.055	1	8.746e-3	2	NC	5	NC	2
4			min	-.761	4	-.126	1	-.028	5	-1.31e-3	3	762.032	3	4862.342	1
5		3	max	.001	1	.601	3	.133	1	9.956e-3	2	NC	5	NC	3
6			min	-.761	4	-.295	1	-.033	5	-1.394e-3	3	421.096	3	1971.392	1
7		4	max	0	1	.768	3	.2	1	1.117e-2	2	NC	5	NC	3
8			min	-.761	4	-.391	1	-.021	5	-1.479e-3	3	331.168	3	1298.646	1
9		5	max	0	1	.806	3	.236	1	1.238e-2	2	NC	5	NC	3
10			min	-.761	4	-.4	1	-.002	5	-1.563e-3	3	315.604	3	1102.797	1
11		6	max	0	1	.719	3	.228	1	1.359e-2	2	NC	5	NC	5
12			min	-.761	4	-.326	1	.012	15	-1.647e-3	3	353.031	3	1139.157	1
13		7	max	0	1	.534	3	.18	1	1.48e-2	2	NC	5	NC	3
14			min	-.761	4	-.186	1	.016	10	-1.731e-3	3	473.384	3	1447.023	1
15		8	max	0	1	.298	3	.105	1	1.601e-2	2	NC	4	NC	3
16			min	-.761	4	-.015	9	.005	10	-1.815e-3	3	834.663	3	2488.256	1
17		9	max	0	1	.155	2	.038	4	1.722e-2	2	NC	4	NC	2
18			min	-.761	4	.004	15	-.005	10	-1.9e-3	3	2709.692	3	6727.881	4
19		10	max	0	1	.22	2	.023	3	1.843e-2	2	NC	3	NC	1
20			min	-.761	4	-.013	3	-.015	2	-1.984e-3	3	1998.721	2	NC	1
21		11	max	0	12	.155	2	.031	1	1.722e-2	2	NC	4	NC	2
22			min	-.761	4	.004	15	-.023	5	-1.9e-3	3	2709.692	3	8831.496	1
23		12	max	0	12	.298	3	.105	1	1.601e-2	2	NC	4	NC	3
24			min	-.761	4	-.015	9	-.022	5	-1.815e-3	3	834.663	3	2488.256	1
25		13	max	0	12	.534	3	.18	1	1.48e-2	2	NC	5	NC	3
26			min	-.761	4	-.186	1	-.006	5	-1.731e-3	3	473.384	3	1447.023	1
27		14	max	0	12	.719	3	.228	1	1.359e-2	2	NC	5	NC	5
28			min	-.761	4	-.326	1	.01	15	-1.647e-3	3	353.031	3	1139.157	1
29		15	max	0	12	.806	3	.236	1	1.238e-2	2	NC	5	NC	3
30			min	-.762	4	-.4	1	.02	12	-1.563e-3	3	315.604	3	1102.797	1
31		16	max	0	12	.768	3	.2	1	1.117e-2	2	NC	5	NC	3
32			min	-.762	4	-.391	1	.017	12	-1.479e-3	3	331.168	3	1298.646	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.601	3	.133	1	9.956e-3	2	NC	5	NC	3
34		min	-762	4	-.295	1	.012	12	-1.394e-3	3	421.096	3	1971.392	1
35	18	max	0	12	.327	3	.055	1	8.746e-3	2	NC	5	NC	2
36		min	-762	4	-.126	1	.004	10	-1.31e-3	3	762.032	3	4862.342	1
37	19	max	0	12	.09	2	.007	3	7.536e-3	2	NC	1	NC	1
38		min	-762	4	-.011	3	-.003	2	-1.226e-3	3	NC	1	NC	1
39	M14	1	max	0	.178	3	.007	3	4.474e-3	2	NC	1	NC	1
40		min	-.561	4	-.296	2	-.003	2	-3.135e-3	3	NC	1	NC	1
41	2	max	0	1	.495	3	.039	1	5.391e-3	2	NC	5	NC	2
42		min	-.561	4	-.604	2	-.041	5	-3.842e-3	3	811.024	1	6100.51	5
43	3	max	0	1	.762	3	.108	1	6.309e-3	2	NC	15	NC	3
44		min	-.561	4	-.875	1	-.048	5	-4.548e-3	3	437.385	1	2438.547	1
45	4	max	0	1	.944	3	.173	1	7.227e-3	2	NC	15	NC	3
46		min	-.561	4	-1.068	1	-.031	5	-5.255e-3	3	329.325	1	1507.623	1
47	5	max	0	1	1.023	3	.21	1	8.145e-3	2	9556.558	15	NC	3
48		min	-.561	4	-1.168	1	-.002	5	-5.961e-3	3	292.262	1	1236.889	1
49	6	max	0	1	1.002	3	.208	1	9.063e-3	2	9561.145	15	NC	3
50		min	-.561	4	-1.173	1	.019	15	-6.668e-3	3	290.617	1	1250.088	1
51	7	max	0	1	.898	3	.167	1	9.98e-3	2	NC	15	NC	3
52		min	-.561	4	-1.103	2	.015	10	-7.374e-3	3	316.916	1	1563.629	1
53	8	max	0	1	.747	3	.099	1	1.09e-2	2	NC	15	NC	3
54		min	-.561	4	-.992	2	.005	10	-8.081e-3	3	370.677	2	2654.965	1
55	9	max	0	1	.602	3	.055	4	1.182e-2	2	NC	15	NC	2
56		min	-.561	4	-.88	2	-.005	10	-8.787e-3	3	441.633	2	4822.669	4
57	10	max	0	1	.535	3	.021	3	1.273e-2	2	NC	5	NC	1
58		min	-.561	4	-.827	2	-.014	2	-9.494e-3	3	485.915	2	NC	1
59	11	max	0	12	.602	3	.03	1	1.182e-2	2	NC	15	NC	2
60		min	-.561	4	-.88	2	-.04	5	-8.787e-3	3	441.633	2	6441.636	5
61	12	max	0	12	.747	3	.099	1	1.09e-2	2	NC	15	NC	3
62		min	-.561	4	-.992	2	-.045	5	-8.081e-3	3	370.677	2	2654.965	1
63	13	max	0	12	.898	3	.167	1	9.98e-3	2	NC	15	NC	3
64		min	-.561	4	-1.103	2	-.027	5	-7.374e-3	3	316.916	1	1563.629	1
65	14	max	0	12	1.002	3	.208	1	9.063e-3	2	9560.773	15	NC	3
66		min	-.561	4	-1.173	1	.001	15	-6.668e-3	3	290.617	1	1250.088	1
67	15	max	0	12	1.023	3	.21	1	8.145e-3	2	9556.093	15	NC	3
68		min	-.561	4	-1.168	1	.018	12	-5.961e-3	3	292.262	1	1236.889	1
69	16	max	0	12	.944	3	.173	1	7.227e-3	2	NC	15	NC	3
70		min	-.561	4	-1.068	1	.015	12	-5.255e-3	3	329.325	1	1507.623	1
71	17	max	0	12	.762	3	.108	1	6.309e-3	2	NC	15	NC	3
72		min	-.561	4	-.875	1	.01	12	-4.548e-3	3	437.385	1	2438.547	1
73	18	max	0	12	.495	3	.057	4	5.391e-3	2	NC	5	NC	2
74		min	-.561	4	-.604	2	.002	10	-3.842e-3	3	811.024	1	4516.154	4
75	19	max	0	12	.178	3	.007	3	4.474e-3	2	NC	1	NC	1
76		min	-.561	4	-.296	2	-.003	2	-3.135e-3	3	NC	1	NC	1
77	M15	1	max	0	.182	3	.006	3	2.705e-3	3	NC	1	NC	1
78		min	-.453	4	-.295	2	-.003	2	-4.671e-3	2	NC	1	NC	1
79	2	max	0	12	.379	3	.039	1	3.321e-3	3	NC	5	NC	2
80		min	-.453	4	-.697	2	-.053	5	-5.633e-3	2	641.7	2	4754.452	5
81	3	max	0	12	.548	3	.108	1	3.936e-3	3	NC	15	NC	3
82		min	-.453	4	-1.037	2	-.063	5	-6.595e-3	2	347.627	2	2432.054	1
83	4	max	0	12	.671	3	.173	1	4.552e-3	3	NC	15	NC	3
84		min	-.453	4	-1.273	2	-.043	5	-7.557e-3	2	263.796	2	1504.527	1
85	5	max	0	12	.738	3	.211	1	5.167e-3	3	9571.817	15	NC	3
86		min	-.453	4	-1.384	2	-.008	5	-8.52e-3	2	236.981	2	1234.605	1
87	6	max	0	12	.748	3	.209	1	5.783e-3	3	9579.509	15	NC	3
88		min	-.453	4	-1.37	2	.019	12	-9.482e-3	2	240.077	2	1247.71	1
89	7	max	0	12	.711	3	.167	1	6.398e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-.453	4	-1.253	2	.015	10	-1.044e-2	2	269.374	2	1560.005	1
91		8	max	0	12	.645	3	.1	4	7.013e-3	3	NC	15	NC	3
92			min	-.453	4	-1.077	2	.005	10	-1.141e-2	2	330.203	2	2630.921	4
93		9	max	0	12	.576	3	.066	4	7.629e-3	3	NC	5	NC	2
94			min	-.453	4	-.905	2	-.004	10	-1.237e-2	2	422.938	2	4007.925	4
95		10	max	0	1	.544	3	.019	3	8.244e-3	3	NC	5	NC	1
96			min	-.453	4	-.825	2	-.013	2	-1.333e-2	2	486.893	2	NC	1
97		11	max	0	1	.576	3	.03	1	7.629e-3	3	NC	5	NC	2
98			min	-.453	4	-.905	2	-.051	5	-1.237e-2	2	422.938	2	5071.263	5
99		12	max	0	1	.645	3	.099	1	7.013e-3	3	NC	15	NC	3
100			min	-.453	4	-1.077	2	-.058	5	-1.141e-2	2	330.203	2	2645.033	1
101		13	max	0	1	.711	3	.167	1	6.398e-3	3	NC	15	NC	3
102			min	-.453	4	-1.253	2	-.037	5	-1.044e-2	2	269.374	2	1560.005	1
103		14	max	0	1	.748	3	.209	1	5.783e-3	3	9579.22	15	NC	3
104			min	-.453	4	-1.37	2	0	15	-9.482e-3	2	240.077	2	1247.71	1
105		15	max	0	1	.738	3	.211	1	5.167e-3	3	9571.458	15	NC	3
106			min	-.453	4	-1.384	2	.017	12	-8.52e-3	2	236.981	2	1234.605	1
107		16	max	0	1	.671	3	.173	1	4.552e-3	3	NC	15	NC	3
108			min	-.453	4	-1.273	2	.014	12	-7.557e-3	2	263.796	2	1504.527	1
109		17	max	0	1	.548	3	.108	1	3.936e-3	3	NC	15	NC	3
110			min	-.453	4	-1.037	2	.01	12	-6.595e-3	2	347.627	2	2432.054	1
111		18	max	0	1	.379	3	.069	4	3.321e-3	3	NC	5	NC	2
112			min	-.453	4	-.697	2	.002	10	-5.633e-3	2	641.7	2	3725.057	4
113		19	max	0	1	.182	3	.006	3	2.705e-3	3	NC	1	NC	1
114			min	-.453	4	-.295	2	-.003	2	-4.671e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.084	1	.005	3	4.706e-3	3	NC	1	NC	1
116			min	-.15	4	-.058	3	-.003	2	-6.428e-3	1	NC	1	NC	1
117		2	max	0	12	.062	3	.054	1	5.606e-3	3	NC	5	NC	2
118			min	-.15	4	-.215	2	-.041	5	-7.401e-3	1	871.529	2	4896.814	1
119		3	max	0	12	.156	3	.132	1	6.507e-3	3	NC	5	NC	3
120			min	-.15	4	-.452	2	-.05	5	-8.374e-3	1	484.278	2	1978.385	1
121		4	max	0	12	.208	3	.2	1	7.407e-3	3	NC	5	NC	3
122			min	-.15	4	-.59	2	-.036	5	-9.347e-3	1	384.809	2	1300.815	1
123		5	max	0	12	.209	3	.235	1	8.307e-3	3	NC	5	NC	3
124			min	-.15	4	-.61	2	-.01	5	-1.032e-2	1	373.442	2	1103.004	1
125		6	max	0	12	.161	3	.228	1	9.207e-3	3	NC	5	NC	3
126			min	-.15	4	-.516	2	.012	15	-1.129e-2	1	432.107	2	1137.473	1
127		7	max	0	12	.076	3	.18	1	1.011e-2	3	NC	5	NC	3
128			min	-.15	4	-.333	2	.017	10	-1.227e-2	1	623.819	2	1441.024	1
129		8	max	0	12	0	5	.106	1	1.101e-2	3	NC	4	NC	3
130			min	-.15	4	-.105	2	.007	10	-1.324e-2	1	1384.919	2	2461.841	1
131		9	max	0	12	.118	1	.049	4	1.191e-2	3	NC	2	NC	2
132			min	-.15	4	-.12	3	-.003	10	-1.421e-2	1	4209.482	3	5304.48	4
133		10	max	0	1	.2	1	.017	3	1.281e-2	3	NC	4	NC	1
134			min	-.15	4	-.16	3	-.012	2	-1.519e-2	1	2210.705	1	NC	1
135		11	max	0	1	.118	1	.032	1	1.191e-2	3	NC	2	NC	2
136			min	-.15	4	-.12	3	-.033	5	-1.421e-2	1	4209.482	3	7735.127	5
137		12	max	0	1	0	15	.106	1	1.101e-2	3	NC	4	NC	3
138			min	-.15	4	-.105	2	-.034	5	-1.324e-2	1	1384.919	2	2461.841	1
139		13	max	0	1	.076	3	.18	1	1.011e-2	3	NC	5	NC	3
140			min	-.15	4	-.333	2	-.015	5	-1.227e-2	1	623.819	2	1441.024	1
141		14	max	0	1	.161	3	.228	1	9.207e-3	3	NC	5	NC	3
142			min	-.15	4	-.516	2	.01	15	-1.129e-2	1	432.107	2	1137.473	1
143		15	max	0	1	.209	3	.235	1	8.307e-3	3	NC	5	NC	3
144			min	-.15	4	-.61	2	.018	12	-1.032e-2	1	373.442	2	1103.004	1
145		16	max	0	1	.208	3	.2	1	7.407e-3	3	NC	5	NC	3
146			min	-.15	4	-.59	2	.015	12	-9.347e-3	1	384.809	2	1300.815	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	.001	1	.156	3	.132	1	6.507e-3	3	NC	5	NC	3
148			min	-.15	4	-.452	2	.011	12	-8.374e-3	1	484.278	2	1978.385	1
149		18	max	.001	1	.062	3	.064	4	5.606e-3	3	NC	5	NC	2
150			min	-.149	4	-.215	2	.004	10	-7.401e-3	1	871.529	2	4052.954	4
151		19	max	.001	1	.084	1	.005	3	4.706e-3	3	NC	1	NC	1
152			min	-.149	4	-.058	3	-.003	2	-6.428e-3	1	NC	1	NC	1
153	M2	1	max	.006	1	.006	2	.01	1	1.621e-3	5	NC	1	NC	2
154			min	-.007	3	-.011	3	-.711	4	-2.883e-4	1	NC	1	98.346	4
155		2	max	.006	1	.005	2	.009	1	1.728e-3	5	NC	1	NC	2
156			min	-.007	3	-.011	3	-.654	4	-2.717e-4	1	NC	1	107.01	4
157		3	max	.005	1	.004	2	.008	1	1.834e-3	5	NC	1	NC	2
158			min	-.006	3	-.01	3	-.596	4	-2.55e-4	1	NC	1	117.284	4
159		4	max	.005	1	.003	2	.008	1	1.94e-3	5	NC	1	NC	2
160			min	-.006	3	-.01	3	-.54	4	-2.384e-4	1	NC	1	129.585	4
161		5	max	.005	1	.002	2	.007	1	2.047e-3	5	NC	1	NC	1
162			min	-.006	3	-.01	3	-.484	4	-2.218e-4	1	NC	1	144.477	4
163		6	max	.004	1	.001	2	.006	1	2.153e-3	5	NC	1	NC	1
164			min	-.005	3	-.009	3	-.43	4	-2.052e-4	1	NC	1	162.741	4
165		7	max	.004	1	0	2	.005	1	2.259e-3	5	NC	1	NC	1
166			min	-.005	3	-.009	3	-.377	4	-1.886e-4	1	NC	1	185.479	4
167		8	max	.004	1	0	2	.004	1	2.366e-3	5	NC	1	NC	1
168			min	-.004	3	-.008	3	-.326	4	-1.72e-4	1	NC	1	214.294	4
169		9	max	.003	1	0	2	.004	1	2.472e-3	5	NC	1	NC	1
170			min	-.004	3	-.008	3	-.278	4	-1.554e-4	1	NC	1	251.589	4
171		10	max	.003	1	0	15	.003	1	2.579e-3	4	NC	1	NC	1
172			min	-.004	3	-.007	3	-.232	4	-1.388e-4	1	NC	1	301.098	4
173		11	max	.003	1	0	15	.003	1	2.691e-3	4	NC	1	NC	1
174			min	-.003	3	-.007	3	-.19	4	-1.222e-4	1	NC	1	368.894	4
175		12	max	.002	1	0	15	.002	1	2.804e-3	4	NC	1	NC	1
176			min	-.003	3	-.006	3	-.15	4	-1.056e-4	1	NC	1	465.404	4
177		13	max	.002	1	0	15	.001	1	2.916e-3	4	NC	1	NC	1
178			min	-.002	3	-.005	3	-.115	4	-8.899e-5	1	NC	1	609.8	4
179		14	max	.002	1	0	15	.001	1	3.028e-3	4	NC	1	NC	1
180			min	-.002	3	-.005	3	-.083	4	-7.238e-5	1	NC	1	840.619	4
181		15	max	.001	1	0	15	0	1	3.141e-3	4	NC	1	NC	1
182			min	-.002	3	-.004	3	-.056	4	-5.577e-5	1	NC	1	1245.272	4
183		16	max	.001	1	0	15	0	1	3.253e-3	4	NC	1	NC	1
184			min	-.001	3	-.003	3	-.034	4	-3.917e-5	1	NC	1	2059.92	4
185		17	max	0	1	0	15	0	1	3.366e-3	4	NC	1	NC	1
186			min	0	3	-.002	6	-.017	4	-2.256e-5	1	NC	1	4129.334	4
187		18	max	0	1	0	15	0	1	3.478e-3	4	NC	1	NC	1
188			min	0	3	-.001	6	-.005	4	-5.956e-6	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	3.59e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	4.546e-7	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-2.008e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-9.039e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.017	4	2.316e-5	1	NC	1	NC	1
194			min	0	2	-.002	6	0	12	-1.823e-4	5	NC	1	NC	1
195		3	max	0	3	0	15	.032	4	5.461e-4	4	NC	1	NC	1
196			min	0	2	-.004	6	0	12	2.704e-6	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.047	4	1.271e-3	4	NC	1	NC	1
198			min	0	2	-.006	6	0	12	4.156e-6	12	NC	1	8884.08	5
199		5	max	.001	3	-.002	15	.06	4	1.996e-3	4	NC	1	NC	1
200			min	-.001	2	-.008	6	0	12	5.608e-6	12	NC	1	8139.37	5
201		6	max	.002	3	-.002	15	.072	4	2.721e-3	4	NC	1	NC	1
202			min	-.001	2	-.009	6	0	12	7.06e-6	12	9764.941	6	8195.231	5
203		7	max	.002	3	-.002	15	.083	4	3.446e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.011	6	0	12	8.512e-6	12	8401.914	6	8952.6	5
205		8	max	.002	3	-.003	15	.094	4	4.171e-3	4	NC	2	NC	1
206			min	-.002	2	-.012	6	0	12	9.964e-6	12	7561.605	6	NC	1
207		9	max	.003	3	-.003	15	.104	4	4.896e-3	4	NC	3	NC	1
208			min	-.002	2	-.013	6	0	12	1.142e-5	12	7066.975	6	NC	1
209		10	max	.003	3	-.003	15	.113	4	5.621e-3	4	NC	5	NC	1
210			min	-.002	2	-.013	6	0	12	1.287e-5	12	6832.325	6	NC	1
211		11	max	.003	3	-.003	15	.123	4	6.346e-3	4	NC	3	NC	1
212			min	-.003	2	-.013	6	0	12	1.432e-5	12	6823.232	6	NC	1
213		12	max	.004	3	-.003	15	.132	4	7.071e-3	4	NC	3	NC	1
214			min	-.003	2	-.013	6	0	12	1.577e-5	12	7043.361	6	NC	1
215		13	max	.004	3	-.003	15	.142	4	7.796e-3	4	NC	2	NC	1
216			min	-.003	2	-.012	6	0	12	1.723e-5	12	7537.577	6	NC	1
217		14	max	.004	3	-.002	15	.153	4	8.521e-3	4	NC	1	NC	1
218			min	-.003	2	-.011	6	0	12	1.868e-5	12	8414.995	6	NC	1
219		15	max	.005	3	-.002	15	.164	4	9.246e-3	4	NC	1	NC	1
220			min	-.004	2	-.009	6	0	12	2.013e-5	12	9916.663	6	NC	1
221		16	max	.005	3	-.001	15	.176	4	9.971e-3	4	NC	1	NC	1
222			min	-.004	2	-.007	1	0	12	2.158e-5	12	NC	1	NC	1
223		17	max	.005	3	0	15	.189	4	1.07e-2	4	NC	1	NC	1
224			min	-.004	2	-.006	1	0	12	2.303e-5	12	NC	1	NC	1
225		18	max	.006	3	0	15	.204	4	1.142e-2	4	NC	1	NC	1
226			min	-.004	2	-.004	1	0	12	2.449e-5	12	NC	1	NC	1
227		19	max	.006	3	0	5	.22	4	1.215e-2	4	NC	1	NC	2
228			min	-.005	2	-.002	1	0	12	2.594e-5	12	NC	1	9455.187	1
229	M4	1	max	.003	1	.004	2	0	12	1.131e-4	1	NC	1	NC	3
230			min	0	5	-.006	3	-.22	4	-8.3e-6	5	NC	1	112.654	4
231		2	max	.002	1	.004	2	0	12	1.131e-4	1	NC	1	NC	3
232			min	0	5	-.006	3	-.203	4	-8.3e-6	5	NC	1	122.423	4
233		3	max	.002	1	.004	2	0	12	1.131e-4	1	NC	1	NC	3
234			min	0	5	-.005	3	-.185	4	-8.3e-6	5	NC	1	134.053	4
235		4	max	.002	1	.004	2	0	12	1.131e-4	1	NC	1	NC	3
236			min	0	5	-.005	3	-.168	4	-8.3e-6	5	NC	1	148.027	4
237		5	max	.002	1	.003	2	0	12	1.131e-4	1	NC	1	NC	3
238			min	0	5	-.005	3	-.15	4	-8.3e-6	5	NC	1	165.001	4
239		6	max	.002	1	.003	2	0	12	1.131e-4	1	NC	1	NC	2
240			min	0	5	-.004	3	-.133	4	-8.3e-6	5	NC	1	185.886	4
241		7	max	.002	1	.003	2	0	12	1.131e-4	1	NC	1	NC	2
242			min	0	5	-.004	3	-.117	4	-8.3e-6	5	NC	1	211.972	4
243		8	max	.002	1	.003	2	0	12	1.131e-4	1	NC	1	NC	2
244			min	0	5	-.004	3	-.101	4	-8.3e-6	5	NC	1	245.142	4
245		9	max	.001	1	.002	2	0	12	1.131e-4	1	NC	1	NC	2
246			min	0	5	-.003	3	-.086	4	-8.3e-6	5	NC	1	288.233	4
247		10	max	.001	1	.002	2	0	12	1.131e-4	1	NC	1	NC	2
248			min	0	5	-.003	3	-.072	4	-8.3e-6	5	NC	1	345.674	4
249		11	max	.001	1	.002	2	0	12	1.131e-4	1	NC	1	NC	2
250			min	0	5	-.003	3	-.058	4	-8.3e-6	5	NC	1	424.715	4
251		12	max	0	1	.002	2	0	12	1.131e-4	1	NC	1	NC	1
252			min	0	5	-.002	3	-.046	4	-8.3e-6	5	NC	1	537.891	4
253		13	max	0	1	.001	2	0	12	1.131e-4	1	NC	1	NC	1
254			min	0	5	-.002	3	-.035	4	-8.3e-6	5	NC	1	708.468	4
255		14	max	0	1	.001	2	0	12	1.131e-4	1	NC	1	NC	1
256			min	0	5	-.002	3	-.025	4	-8.3e-6	5	NC	1	983.766	4
257		15	max	0	1	0	2	0	12	1.131e-4	1	NC	1	NC	1
258			min	0	5	-.001	3	-.017	4	-8.3e-6	5	NC	1	1472.842	4
259		16	max	0	1	0	2	0	12	1.131e-4	1	NC	1	NC	1
260			min	0	5	-.001	3	-.01	4	-8.3e-6	5	NC	1	2477.073	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	1.131e-4	1	NC	1	NC	1
262			min	0	5	0	3	-0.005	4	-8.3e-6	5	NC	1	5113.402	4
263		18	max	0	1	0	2	0	12	1.131e-4	1	NC	1	NC	1
264			min	0	5	0	3	-0.001	4	-8.3e-6	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.131e-4	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.3e-6	5	NC	1	NC	1
267	M6	1	max	.02	1	.025	2	0	1	1.729e-3	4	NC	3	NC	1
268			min	-.024	3	-.034	3	-.718	4	0	1	2854.669	2	97.402	4
269		2	max	.018	1	.022	2	0	1	1.833e-3	4	NC	3	NC	1
270			min	-.022	3	-.032	3	-.66	4	0	1	3139.639	2	105.983	4
271		3	max	.017	1	.02	2	0	1	1.937e-3	4	NC	3	NC	1
272			min	-.021	3	-.031	3	-.602	4	0	1	3484.634	2	116.161	4
273		4	max	.016	1	.018	2	0	1	2.041e-3	4	NC	3	NC	1
274			min	-.02	3	-.029	3	-.545	4	0	1	3906.92	2	128.347	4
275		5	max	.015	1	.016	2	0	1	2.145e-3	4	NC	3	NC	1
276			min	-.018	3	-.027	3	-.489	4	0	1	4430.672	2	143.1	4
277		6	max	.014	1	.014	2	0	1	2.249e-3	4	NC	1	NC	1
278			min	-.017	3	-.025	3	-.434	4	0	1	5090.573	2	161.194	4
279		7	max	.013	1	.012	2	0	1	2.353e-3	4	NC	1	NC	1
280			min	-.016	3	-.023	3	-.381	4	0	1	5937.825	2	183.722	4
281		8	max	.012	1	.01	2	0	1	2.457e-3	4	NC	1	NC	1
282			min	-.014	3	-.021	3	-.33	4	0	1	7050.635	2	212.272	4
283		9	max	.011	1	.008	2	0	1	2.561e-3	4	NC	1	NC	1
284			min	-.013	3	-.019	3	-.281	4	0	1	8553.418	2	249.225	4
285		10	max	.01	1	.007	2	0	1	2.665e-3	4	NC	1	NC	1
286			min	-.012	3	-.017	3	-.235	4	0	1	NC	1	298.284	4
287		11	max	.009	1	.005	2	0	1	2.769e-3	4	NC	1	NC	1
288			min	-.01	3	-.015	3	-.191	4	0	1	NC	1	365.469	4
289		12	max	.008	1	.004	2	0	1	2.873e-3	4	NC	1	NC	1
290			min	-.009	3	-.014	3	-.152	4	0	1	NC	1	461.116	4
291		13	max	.007	1	.003	2	0	1	2.977e-3	4	NC	1	NC	1
292			min	-.008	3	-.012	3	-.116	4	0	1	NC	1	604.236	4
293		14	max	.005	1	.002	2	0	1	3.081e-3	4	NC	1	NC	1
294			min	-.007	3	-.01	3	-.084	4	0	1	NC	1	833.046	4
295		15	max	.004	1	0	2	0	1	3.185e-3	4	NC	1	NC	1
296			min	-.005	3	-.008	3	-.057	4	0	1	NC	1	1234.255	4
297		16	max	.003	1	0	2	0	1	3.289e-3	4	NC	1	NC	1
298			min	-.004	3	-.006	3	-.034	4	0	1	NC	1	2042.192	4
299		17	max	.002	1	0	2	0	1	3.393e-3	4	NC	1	NC	1
300			min	-.003	3	-.004	3	-.017	4	0	1	NC	1	4095.478	4
301		18	max	.001	1	0	2	0	1	3.497e-3	4	NC	1	NC	1
302			min	-.001	3	-.002	3	-.006	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	3.601e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-9.059e-4	4	NC	1	NC	1
307		2	max	.001	3	0	15	.017	4	0	1	NC	1	NC	1
308			min	-.001	2	-.002	3	0	1	-2.006e-4	4	NC	1	NC	1
309		3	max	.002	3	0	15	.033	4	5.047e-4	4	NC	1	NC	1
310			min	-.002	2	-.005	3	0	1	0	1	NC	1	NC	1
311		4	max	.003	3	-.001	15	.047	4	1.21e-3	4	NC	1	NC	1
312			min	-.003	2	-.007	3	0	1	0	1	NC	1	8017.241	4
313		5	max	.004	3	-.002	15	.06	4	1.915e-3	4	NC	1	NC	1
314			min	-.004	2	-.009	3	0	1	0	1	NC	1	7235.157	4
315		6	max	.005	3	-.002	15	.072	4	2.621e-3	4	NC	1	NC	1
316			min	-.005	2	-.011	3	0	1	0	1	9736.356	3	7138.424	4
317		7	max	.006	3	-.003	15	.083	4	3.326e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.006	2	-.012	3	0	1	0	1	8474.866	4	7576.995	4
319	8	max	.007	3	-.003	15	.093	4	4.031e-3	4	NC	1	NC	1
320		min	-.007	2	-.013	3	0	1	0	1	7622.654	4	8648.452	4
321	9	max	.008	3	-.003	15	.103	4	4.737e-3	4	NC	1	NC	1
322		min	-.008	2	-.014	3	0	1	0	1	7120.439	4	NC	1
323	10	max	.009	3	-.003	15	.112	4	5.442e-3	4	NC	1	NC	1
324		min	-.009	2	-.014	3	0	1	0	1	6881.094	4	NC	1
325	11	max	.01	3	-.003	15	.122	4	6.147e-3	4	NC	1	NC	1
326		min	-.01	2	-.015	3	0	1	0	1	6869.475	4	NC	1
327	12	max	.011	3	-.003	15	.131	4	6.853e-3	4	NC	1	NC	1
328		min	-.011	2	-.014	3	0	1	0	1	7088.95	4	NC	1
329	13	max	.012	3	-.003	15	.14	4	7.558e-3	4	NC	1	NC	1
330		min	-.012	2	-.014	3	0	1	0	1	7584.433	4	NC	1
331	14	max	.013	3	-.003	15	.15	4	8.263e-3	4	NC	1	NC	1
332		min	-.013	2	-.013	3	0	1	0	1	8465.51	4	NC	1
333	15	max	.014	3	-.002	15	.16	4	8.969e-3	4	NC	1	NC	1
334		min	-.014	2	-.012	3	0	1	0	1	9974.464	4	NC	1
335	16	max	.015	3	-.002	15	.171	4	9.674e-3	4	NC	1	NC	1
336		min	-.015	2	-.01	3	0	1	0	1	NC	1	NC	1
337	17	max	.017	3	-.001	15	.184	4	1.038e-2	4	NC	1	NC	1
338		min	-.016	2	-.009	3	0	1	0	1	NC	1	NC	1
339	18	max	.018	3	0	15	.197	4	1.108e-2	4	NC	1	NC	1
340		min	-.017	2	-.007	3	0	1	0	1	NC	1	NC	1
341	19	max	.019	3	0	10	.213	4	1.179e-2	4	NC	1	NC	1
342		min	-.018	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	1	.017	2	0	0	1	NC	1	NC	1
344		min	0	3	-.019	3	-.213	4	-1.13e-4	4	NC	1	116.522	4
345	2	max	.006	1	.016	2	0	1	0	1	NC	1	NC	1
346		min	0	3	-.018	3	-.196	4	-1.13e-4	4	NC	1	126.635	4
347	3	max	.006	1	.015	2	0	1	0	1	NC	1	NC	1
348		min	0	3	-.017	3	-.179	4	-1.13e-4	4	NC	1	138.674	4
349	4	max	.005	1	.014	2	0	1	0	1	NC	1	NC	1
350		min	0	3	-.016	3	-.162	4	-1.13e-4	4	NC	1	153.14	4
351	5	max	.005	1	.013	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.015	3	-.145	4	-1.13e-4	4	NC	1	170.711	4
353	6	max	.005	1	.012	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.014	3	-.129	4	-1.13e-4	4	NC	1	192.329	4
355	7	max	.004	1	.012	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.013	3	-.113	4	-1.13e-4	4	NC	1	219.331	4
357	8	max	.004	1	.011	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.012	3	-.098	4	-1.13e-4	4	NC	1	253.666	4
359	9	max	.004	1	.01	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.011	3	-.083	4	-1.13e-4	4	NC	1	298.269	4
361	10	max	.003	1	.009	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.01	3	-.069	4	-1.13e-4	4	NC	1	357.728	4
363	11	max	.003	1	.008	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.009	3	-.056	4	-1.13e-4	4	NC	1	439.544	4
365	12	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.007	3	-.045	4	-1.13e-4	4	NC	1	556.697	4
367	13	max	.002	1	.006	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.006	3	-.034	4	-1.13e-4	4	NC	1	733.268	4
369	14	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.005	3	-.024	4	-1.13e-4	4	NC	1	1018.245	4
371	15	max	.001	1	.004	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.004	3	-.016	4	-1.13e-4	4	NC	1	1524.526	4
373	16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.003	3	-.01	4	-1.13e-4	4	NC	1	2564.11	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.005	4	-1.13e-4	4	NC	1	5293.344	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-1.13e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-1.13e-4	4	NC	1	NC	1
381	M10	1	max	.006	1	.006	2	0	12	1.745e-3	4	NC	1	NC	2
382			min	-.007	3	-.011	3	-.717	4	1.677e-5	12	NC	1	97.498	4
383		2	max	.006	1	.005	2	0	12	1.847e-3	4	NC	1	NC	2
384			min	-.007	3	-.011	3	-.659	4	1.582e-5	12	NC	1	106.089	4
385		3	max	.005	1	.004	2	0	12	1.949e-3	4	NC	1	NC	2
386			min	-.006	3	-.01	3	-.602	4	1.486e-5	12	NC	1	116.277	4
387		4	max	.005	1	.003	2	0	12	2.052e-3	4	NC	1	NC	2
388			min	-.006	3	-.01	3	-.544	4	1.39e-5	12	NC	1	128.476	4
389		5	max	.005	1	.002	2	0	12	2.154e-3	4	NC	1	NC	1
390			min	-.006	3	-.01	3	-.488	4	1.295e-5	12	NC	1	143.245	4
391		6	max	.004	1	.001	2	0	12	2.256e-3	4	NC	1	NC	1
392			min	-.005	3	-.009	3	-.433	4	1.199e-5	12	NC	1	161.359	4
393		7	max	.004	1	0	2	0	12	2.358e-3	4	NC	1	NC	1
394			min	-.005	3	-.009	3	-.38	4	1.103e-5	12	NC	1	183.912	4
395		8	max	.004	1	0	2	0	12	2.46e-3	4	NC	1	NC	1
396			min	-.004	3	-.008	3	-.329	4	1.007e-5	12	NC	1	212.494	4
397		9	max	.003	1	0	2	0	12	2.562e-3	4	NC	1	NC	1
398			min	-.004	3	-.008	3	-.28	4	9.117e-6	12	NC	1	249.49	4
399		10	max	.003	1	-.001	2	0	12	2.664e-3	4	NC	1	NC	1
400			min	-.004	3	-.007	3	-.234	4	8.16e-6	12	NC	1	298.607	4
401		11	max	.003	1	-.002	2	0	12	2.766e-3	4	NC	1	NC	1
402			min	-.003	3	-.007	3	-.191	4	7.203e-6	12	NC	1	365.874	4
403		12	max	.002	1	-.001	15	0	12	2.869e-3	4	NC	1	NC	1
404			min	-.003	3	-.006	3	-.152	4	6.246e-6	12	NC	1	461.643	4
405		13	max	.002	1	-.001	15	0	12	2.971e-3	4	NC	1	NC	1
406			min	-.002	3	-.005	3	-.116	4	5.289e-6	12	NC	1	604.953	4
407		14	max	.002	1	-.001	15	0	12	3.073e-3	4	NC	1	NC	1
408			min	-.002	3	-.005	4	-.084	4	4.331e-6	12	NC	1	834.085	4
409		15	max	.001	1	-.001	15	0	12	3.175e-3	4	NC	1	NC	1
410			min	-.002	3	-.004	4	-.057	4	3.374e-6	12	NC	1	1235.901	4
411		16	max	.001	1	0	15	0	12	3.277e-3	4	NC	1	NC	1
412			min	-.001	3	-.003	4	-.034	4	2.417e-6	12	NC	1	2045.19	4
413		17	max	0	1	0	15	0	12	3.379e-3	4	NC	1	NC	1
414			min	0	3	-.002	4	-.017	4	1.46e-6	12	NC	1	4102.449	4
415		18	max	0	1	0	15	0	12	3.481e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.006	4	5.026e-7	12	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.584e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.065e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	4.323e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-9.012e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.017	4	-1.251e-6	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.931e-4	4	NC	1	NC	1
423		3	max	0	3	0	15	.032	4	5.179e-4	5	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-5.063e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.046	4	1.223e-3	4	NC	1	NC	1
426			min	0	2	-.006	4	0	1	-7.811e-5	1	NC	1	8369.626	4
427		5	max	.001	3	-.002	15	.059	4	1.931e-3	4	NC	1	NC	1
428			min	-.001	2	-.008	4	0	1	-1.056e-4	1	NC	1	7611.225	4
429		6	max	.002	3	-.002	15	.071	4	2.639e-3	4	NC	1	NC	1
430			min	-.001	2	-.01	4	0	1	-1.331e-4	1	9484.092	4	7586.148	4
431		7	max	.002	3	-.003	15	.082	4	3.347e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.012	4	0	1	-1.605e-4	1	8178.536	4	8167.098	4
433		8	max	.002	3	-.003	15	.093	4	4.055e-3	4	NC	2	NC	1
434			min	-.002	2	-.013	4	-.001	1	-1.88e-4	1	7374.212	4	9521.503	4
435		9	max	.003	3	-.003	15	.103	4	4.763e-3	4	NC	3	NC	1
436			min	-.002	2	-.014	4	-.002	1	-2.155e-4	1	6902.529	4	NC	1
437		10	max	.003	3	-.003	15	.112	4	5.471e-3	4	NC	5	NC	1
438			min	-.002	2	-.014	4	-.002	1	-2.43e-4	1	6682.059	4	NC	1
439		11	max	.003	3	-.004	15	.121	4	6.179e-3	4	NC	3	NC	1
440			min	-.003	2	-.014	4	-.002	1	-2.705e-4	1	6680.54	4	NC	1
441		12	max	.004	3	-.003	15	.13	4	6.887e-3	4	NC	3	NC	1
442			min	-.003	2	-.014	4	-.003	1	-2.979e-4	1	6902.512	4	NC	1
443		13	max	.004	3	-.003	15	.14	4	7.595e-3	4	NC	2	NC	1
444			min	-.003	2	-.013	4	-.004	1	-3.254e-4	1	7392.663	4	NC	1
445		14	max	.004	3	-.003	15	.15	4	8.303e-3	4	NC	1	NC	1
446			min	-.003	2	-.012	4	-.004	1	-3.529e-4	1	8258.631	4	NC	1
447		15	max	.005	3	-.003	15	.16	4	9.011e-3	4	NC	1	NC	1
448			min	-.004	2	-.01	4	-.005	1	-3.804e-4	1	9737.618	4	NC	1
449		16	max	.005	3	-.002	15	.172	4	9.719e-3	4	NC	1	NC	1
450			min	-.004	2	-.008	4	-.006	1	-4.079e-4	1	NC	1	NC	1
451		17	max	.005	3	-.002	15	.184	4	1.043e-2	4	NC	1	NC	1
452			min	-.004	2	-.006	4	-.007	1	-4.353e-4	1	NC	1	NC	1
453		18	max	.006	3	-.001	15	.199	4	1.114e-2	4	NC	1	NC	1
454			min	-.004	2	-.004	1	-.008	1	-4.628e-4	1	NC	1	NC	1
455		19	max	.006	3	0	10	.214	4	1.184e-2	4	NC	1	NC	2
456			min	-.005	2	-.002	1	-.01	1	-4.903e-4	1	NC	1	9455.187	1
457	M12	1	max	.003	1	.004	2	.01	1	-6.225e-6	12	NC	1	NC	3
458			min	0	3	-.006	3	-.214	4	-1.131e-4	1	NC	1	115.748	4
459		2	max	.002	1	.004	2	.009	1	-6.225e-6	12	NC	1	NC	3
460			min	0	3	-.006	3	-.197	4	-1.131e-4	1	NC	1	125.788	4
461		3	max	.002	1	.004	2	.008	1	-6.225e-6	12	NC	1	NC	3
462			min	0	3	-.005	3	-.18	4	-1.131e-4	1	NC	1	137.74	4
463		4	max	.002	1	.004	2	.007	1	-6.225e-6	12	NC	1	NC	3
464			min	0	3	-.005	3	-.163	4	-1.131e-4	1	NC	1	152.101	4
465		5	max	.002	1	.003	2	.007	1	-6.225e-6	12	NC	1	NC	3
466			min	0	3	-.005	3	-.146	4	-1.131e-4	1	NC	1	169.546	4
467		6	max	.002	1	.003	2	.006	1	-6.225e-6	12	NC	1	NC	2
468			min	0	3	-.004	3	-.13	4	-1.131e-4	1	NC	1	191.008	4
469		7	max	.002	1	.003	2	.005	1	-6.225e-6	12	NC	1	NC	2
470			min	0	3	-.004	3	-.114	4	-1.131e-4	1	NC	1	217.816	4
471		8	max	.002	1	.003	2	.004	1	-6.225e-6	12	NC	1	NC	2
472			min	0	3	-.004	3	-.098	4	-1.131e-4	1	NC	1	251.905	4
473		9	max	.001	1	.002	2	.004	1	-6.225e-6	12	NC	1	NC	2
474			min	0	3	-.003	3	-.084	4	-1.131e-4	1	NC	1	296.188	4
475		10	max	.001	1	.002	2	.003	1	-6.225e-6	12	NC	1	NC	2
476			min	0	3	-.003	3	-.07	4	-1.131e-4	1	NC	1	355.219	4
477		11	max	.001	1	.002	2	.003	1	-6.225e-6	12	NC	1	NC	2
478			min	0	3	-.003	3	-.057	4	-1.131e-4	1	NC	1	436.447	4
479		12	max	0	1	.002	2	.002	1	-6.225e-6	12	NC	1	NC	1
480			min	0	3	-.002	3	-.045	4	-1.131e-4	1	NC	1	552.757	4
481		13	max	0	1	.001	2	.002	1	-6.225e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.034	4	-1.131e-4	1	NC	1	728.056	4
483		14	max	0	1	.001	2	.001	1	-6.225e-6	12	NC	1	NC	1
484			min	0	3	-.002	3	-.025	4	-1.131e-4	1	NC	1	1010.976	4
485		15	max	0	1	0	2	0	1	-6.225e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.016	4	-1.131e-4	1	NC	1	1513.595	4
487		16	max	0	1	0	2	0	1	-6.225e-6	12	NC	1	NC	1
488			min	0	3	-.001	3	-.01	4	-1.131e-4	1	NC	1	2545.643	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	-6.225e-6	12	NC	1	NC	1
490		min	0	3	0	3	-.005	4	-1.131e-4	1	NC	1	5255.022	4
491	18	max	0	1	0	2	0	1	-6.225e-6	12	NC	1	NC	1
492		min	0	3	0	3	-.001	4	-1.131e-4	1	NC	1	NC	1
493	19	max	0	1	0	1	0	1	-6.225e-6	12	NC	1	NC	1
494		min	0	1	0	1	0	1	-1.131e-4	1	NC	1	NC	1
495	M1	1	max	.007	3	.09	.762	4	1.542e-2	1	NC	1	NC	1
496		min	-.003	2	-.011	3	0	12	-2.45e-2	3	NC	1	NC	1
497	2	max	.007	3	.043	2	.736	4	8.821e-3	4	NC	3	NC	1
498		min	-.003	2	-.003	3	-.007	1	-1.212e-2	3	2404.92	2	NC	1
499	3	max	.007	3	.011	3	.711	4	1.429e-2	4	NC	5	NC	1
500		min	-.003	2	-.009	2	-.01	1	-2.099e-4	1	1157.386	2	5885.531	5
501	4	max	.007	3	.037	3	.684	4	1.249e-2	4	NC	5	NC	1
502		min	-.003	2	-.067	2	-.009	1	-4.326e-3	3	729.117	2	4227.211	5
503	5	max	.007	3	.07	3	.658	4	1.068e-2	4	NC	5	NC	1
504		min	-.003	2	-.129	2	-.007	1	-8.53e-3	3	525.31	2	3397.459	5
505	6	max	.007	3	.106	3	.63	4	1.259e-2	1	NC	15	NC	1
506		min	-.003	2	-.188	2	-.003	1	-1.273e-2	3	413.19	2	2899.647	5
507	7	max	.007	3	.14	3	.602	4	1.685e-2	1	NC	15	NC	1
508		min	-.003	2	-.241	2	0	12	-1.694e-2	3	347.085	2	2548.184	4
509	8	max	.007	3	.169	3	.573	4	2.112e-2	1	9195.526	15	NC	1
510		min	-.003	2	-.283	2	0	12	-2.114e-2	3	308.02	2	2292.405	4
511	9	max	.007	3	.188	3	.543	4	2.346e-2	1	8590.625	15	NC	1
512		min	-.003	2	-.31	2	0	1	-2.118e-2	3	287.699	2	2135.126	4
513	10	max	.006	3	.195	3	.51	4	2.549e-2	2	8406.489	15	NC	1
514		min	-.003	2	-.319	2	0	12	-1.847e-2	3	281.736	2	2090.509	4
515	11	max	.006	3	.19	3	.475	4	2.761e-2	2	8590.312	15	NC	1
516		min	-.003	2	-.31	2	0	12	-1.575e-2	3	288.66	2	2140.008	4
517	12	max	.006	3	.174	3	.437	4	2.678e-2	2	9194.817	15	NC	1
518		min	-.003	2	-.282	2	-.001	1	-1.308e-2	3	310.968	2	2297.343	4
519	13	max	.006	3	.148	3	.396	4	2.149e-2	2	NC	15	NC	1
520		min	-.003	2	-.238	2	0	1	-1.047e-2	3	354.305	2	2693.905	4
521	14	max	.006	3	.115	3	.352	4	1.62e-2	2	NC	15	NC	1
522		min	-.003	2	-.182	2	0	12	-7.855e-3	3	428.683	2	3512.427	4
523	15	max	.006	3	.078	3	.307	4	1.091e-2	2	NC	5	NC	1
524		min	-.003	2	-.121	2	0	12	-5.243e-3	3	557.326	2	5266.216	4
525	16	max	.005	3	.04	3	.263	4	9.747e-3	4	NC	5	NC	1
526		min	-.003	2	-.061	2	0	12	-2.63e-3	3	796.93	2	9901.142	4
527	17	max	.005	3	.004	3	.221	4	1.093e-2	4	NC	5	NC	1
528		min	-.003	2	-.005	2	0	12	-1.723e-5	3	1307.538	1	NC	1
529	18	max	.005	3	.043	1	.183	4	1.072e-2	2	NC	4	NC	1
530		min	-.003	2	-.028	3	0	12	-4.291e-3	3	2768.594	1	NC	1
531	19	max	.005	3	.084	1	.149	4	2.146e-2	2	NC	1	NC	1
532		min	-.003	2	-.058	3	-.001	1	-8.726e-3	3	NC	1	NC	1
533	M5	1	max	.023	3	.22	.761	4	0	1	NC	1	NC	1
534		min	-.015	2	-.013	3	0	1	-5.059e-6	4	NC	1	NC	1
535	2	max	.023	3	.102	2	.741	4	7.343e-3	4	NC	5	NC	1
536		min	-.016	2	.001	3	0	1	0	1	979.642	2	8076.64	4
537	3	max	.023	3	.036	3	.718	4	1.446e-2	4	NC	5	NC	1
538		min	-.016	2	-.03	2	0	1	0	1	462.568	2	4719.83	4
539	4	max	.023	3	.108	3	.691	4	1.178e-2	4	9645.792	15	NC	1
540		min	-.015	2	-.186	2	0	1	0	1	284.475	2	3642.64	4
541	5	max	.022	3	.205	3	.662	4	9.104e-3	4	6754.804	15	NC	1
542		min	-.015	2	-.354	2	0	1	0	1	200.992	2	3128.251	4
543	6	max	.022	3	.313	3	.632	4	6.425e-3	4	5203.086	15	NC	1
544		min	-.015	2	-.52	2	0	1	0	1	155.798	2	2816.283	4
545	7	max	.021	3	.419	3	.602	4	3.745e-3	4	4306.455	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.014	2	-.67	2	0	1	0	1	129.505	2	2573.122	4
547	8	max	.021	3	.507	3	.572	4	1.066e-3	4	3784.885	15	NC	1
548		min	-.014	2	-.791	2	0	1	0	1	114.135	2	2334.574	4
549	9	max	.02	3	.564	3	.543	4	0	1	3517.327	15	NC	1
550		min	-.014	2	-.867	2	0	1	-3.569e-6	5	106.222	2	2130.376	4
551	10	max	.02	3	.584	3	.51	4	0	1	3436.712	15	NC	1
552		min	-.014	2	-.892	2	0	1	-3.46e-6	5	103.905	2	2103.634	4
553	11	max	.019	3	.569	3	.474	4	0	1	3517.429	15	NC	1
554		min	-.013	2	-.866	2	0	1	-3.352e-6	5	106.589	2	2164.379	4
555	12	max	.019	3	.52	3	.438	4	7.726e-4	4	3785.132	15	NC	1
556		min	-.013	2	-.787	2	0	1	0	1	115.329	2	2254.944	4
557	13	max	.018	3	.441	3	.397	4	2.716e-3	4	4306.971	15	NC	1
558		min	-.013	2	-.66	2	0	1	0	1	132.585	2	2654.139	4
559	14	max	.018	3	.342	3	.351	4	4.66e-3	4	5204.114	15	NC	1
560		min	-.013	2	-.502	2	0	1	0	1	162.701	2	3697.115	4
561	15	max	.017	3	.231	3	.303	4	6.604e-3	4	6756.861	15	NC	1
562		min	-.013	2	-.331	2	0	1	0	1	215.935	2	6697.8	4
563	16	max	.017	3	.118	3	.256	4	8.547e-3	4	9650.132	15	NC	1
564		min	-.013	2	-.164	2	0	1	0	1	316.648	1	NC	1
565	17	max	.017	3	.012	3	.214	4	1.049e-2	4	NC	5	NC	1
566		min	-.012	2	-.016	2	0	1	0	1	533.673	1	NC	1
567	18	max	.017	3	.103	1	.178	4	5.327e-3	4	NC	5	NC	1
568		min	-.012	2	-.078	3	0	1	0	1	1161.455	1	NC	1
569	19	max	.017	3	.2	1	.15	4	0	1	NC	1	NC	1
570		min	-.012	2	-.16	3	0	1	-3.006e-6	4	NC	1	NC	1
571	M9	1	max	.007	3	.09	.761	4	2.45e-2	3	NC	1	NC	1
572		min	-.003	2	-.011	3	-.001	1	-1.542e-2	1	NC	1	NC	1
573	2	max	.007	3	.043	2	.741	4	1.212e-2	3	NC	3	NC	1
574		min	-.003	2	-.003	3	0	12	-7.466e-3	1	2404.92	2	8259.054	4
575	3	max	.007	3	.011	3	.717	4	1.444e-2	4	NC	5	NC	1
576		min	-.003	2	-.009	2	0	12	-1.22e-5	10	1157.386	2	4782.98	4
577	4	max	.007	3	.037	3	.69	4	1.132e-2	5	NC	5	NC	1
578		min	-.003	2	-.067	2	0	12	-4.064e-3	2	729.117	2	3655.822	4
579	5	max	.007	3	.07	3	.662	4	8.53e-3	3	NC	5	NC	1
580		min	-.003	2	-.129	2	0	12	-8.321e-3	1	525.31	2	3112.569	4
581	6	max	.007	3	.106	3	.632	4	1.273e-2	3	NC	15	NC	1
582		min	-.003	2	-.188	2	0	12	-1.259e-2	1	413.19	2	2785.711	4
583	7	max	.007	3	.14	3	.602	4	1.694e-2	3	NC	15	NC	1
584		min	-.003	2	-.241	2	0	1	-1.685e-2	1	347.085	2	2541.972	4
585	8	max	.007	3	.169	3	.573	4	2.114e-2	3	9174.255	15	NC	1
586		min	-.003	2	-.283	2	-.001	1	-2.112e-2	1	308.02	2	2317.797	4
587	9	max	.007	3	.188	3	.543	4	2.118e-2	3	8571.023	15	NC	1
588		min	-.003	2	-.31	2	0	12	-2.346e-2	1	287.699	2	2128.22	4
589	10	max	.006	3	.195	3	.51	4	1.847e-2	3	8387.38	15	NC	1
590		min	-.003	2	-.319	2	0	1	-2.549e-2	2	281.736	2	2091.886	4
591	11	max	.006	3	.19	3	.475	4	1.575e-2	3	8570.733	15	NC	1
592		min	-.003	2	-.31	2	0	1	-2.761e-2	2	288.66	2	2149.26	4
593	12	max	.006	3	.174	3	.438	4	1.308e-2	3	9173.705	15	NC	1
594		min	-.003	2	-.282	2	0	12	-2.678e-2	2	310.968	2	2272.766	4
595	13	max	.006	3	.148	3	.396	4	1.047e-2	3	NC	15	NC	1
596		min	-.003	2	-.238	2	0	12	-2.149e-2	2	354.305	2	2696.604	4
597	14	max	.006	3	.115	3	.35	4	7.855e-3	3	NC	15	NC	1
598		min	-.003	2	-.182	2	-.002	1	-1.62e-2	2	428.683	2	3666.824	5
599	15	max	.006	3	.078	3	.303	4	6.23e-3	5	NC	5	NC	1
600		min	-.003	2	-.121	2	-.006	1	-1.091e-2	2	557.326	2	6009.299	5
601	16	max	.005	3	.04	3	.257	4	8.385e-3	5	NC	5	NC	1
602		min	-.003	2	-.061	2	-.009	1	-5.626e-3	2	796.93	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.005	3	.004	3	.215	4	1.057e-2	4	NC	5	NC	1
604		min	-.003	2	-.005	2	-.01	1	-6.342e-4	1	1307.538	1	NC	1
605	18	max	.005	3	.043	1	.179	4	4.981e-3	5	NC	4	NC	1
606		min	-.003	2	-.028	3	-.007	1	-1.072e-2	2	2768.594	1	NC	1
607	19	max	.005	3	.084	1	.15	4	8.726e-3	3	NC	1	NC	1
608		min	-.003	2	-.058	3	0	12	-2.146e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298





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Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Software  
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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

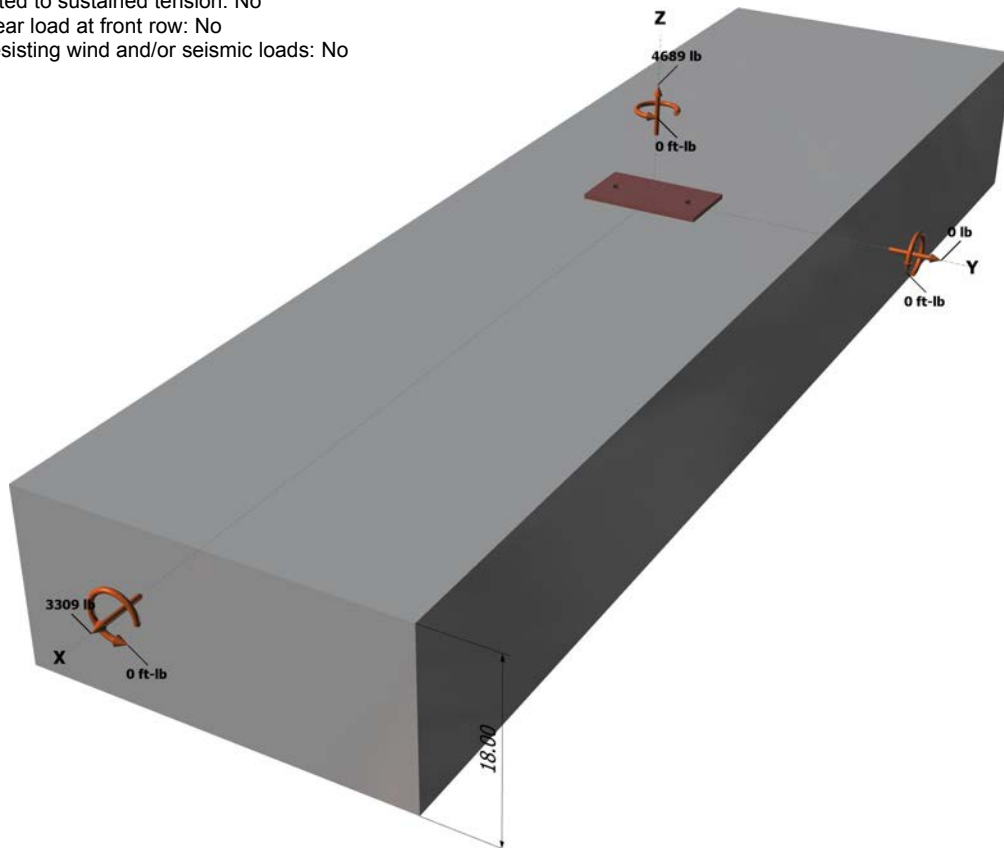
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

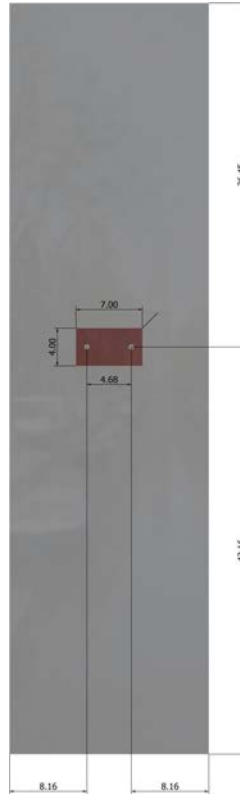
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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
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Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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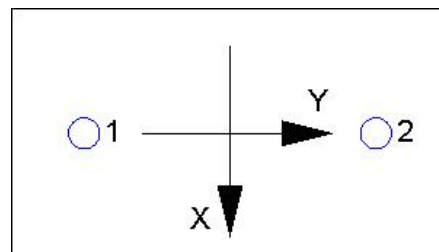
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.