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|-----------------|--|-----------------------------|
| Schletter, Inc. | Standard PVMax Racking System Representative Calculations - ASCE 7-10 | 30° Tilt w/o Seismic Design |
| HCV | | |
| | | |

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

| | Maximum | | Minimum |
|-------------|----------|-------------|----------|
| Height = | 2000 mm | Height = | 1900 mm |
| Width = | 1050 mm | Width = | 970 mm |
| Dead Load = | 3.00 psf | Dead Load = | 1.75 psf |

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

| | |
|-------------|----------|
| g_{MAX} = | 3.00 psf |
| g_{MIN} = | 1.75 psf |

Self-weight of the PV modules.

2.2 Snow Loads

| | | |
|--------------------------------|-----------|------------------------|
| Ground Snow Load, P_g = | 30.00 psf | |
| Sloped Roof Snow Load, P_s = | 16.49 psf | (ASCE 7-10, Eq. 7.4-1) |
| I_s = | 1.00 | |
| C_s = | 0.73 | |
| C_e = | 0.90 | |
| C_t = | 1.20 | |

2.3 Wind Loads

| | | |
|--------------------------|---------|--------------------------|
| Design Wind Speed, V = | 160 mph | Exposure Category = C |
| Height < | 15 ft | Importance Category = II |

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

| | | | |
|----------------------------|---|--------|------------|
| C_{f+} TOP | = | 1.150 | (Pressure) |
| C_{f+} BOTTOM | = | 1.850 | |
| C_{f-} TOP, OUTER PURLIN | = | -2.600 | |
| C_{f-} TOP, INNER PURLIN | = | -2.000 | (Suction) |
| C_{f-} BOTTOM | = | -1.100 | |

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

| | | | |
|------------|------|------------|------|
| S_S = | 0.00 | R = | 1.25 |
| S_{DS} = | 0.00 | C_s = | 0 |
| S_1 = | 0.00 | ρ = | 1.3 |
| S_{D1} = | 0.00 | Ω = | 1.25 |
| T_a = | 0.00 | C_d = | 1.25 |

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

| <u>Purlins</u> | <u>Location</u> | <u>Diagonal Struts</u> | <u>Location</u> | <u>Front Reactions</u> | <u>Location</u> |
|---------------------|-----------------|------------------------|-----------------|------------------------|-----------------|
| M13 | Top | M3 | Outer | N7 | Outer |
| M14 | Mid-Top | M7 | Inner | N15 | Inner |
| M15 | Mid-Bottom | M11 | Outer | N23 | Outer |
| M16 | Bottom | | | | |
| <u>Girders</u> | <u>Location</u> | <u>Rear Struts</u> | <u>Location</u> | <u>Rear Reactions</u> | <u>Location</u> |
| M1 | Outer | M2 | Outer | N8 | Outer |
| M5 | Inner | M6 | Inner | N16 | Inner |
| M9 | Outer | M10 | Outer | N24 | Outer |
| <u>Front Struts</u> | <u>Location</u> | | | | |
| M4 | Outer | | | | |
| M8 | Inner | | | | |
| M12 | Outer | | | | |

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

| | |
|-----------------------------|----------------------|
| Purlin Type = | S1.5 |
| Aluminum Type = | 6105-T5 |
| F_{ty} = | 35 ksi |
| L_b = | 75 in |
| ΦF_{ty} STRONG-AXIS = | 25.07 ksi |
| ΦF_{ty} WEAK-AXIS = | 23.08 ksi |
| S_y = | 1.33 in ³ |
| S_x = | 0.60 in ³ |
| E = | 10100 ksi |
| I_y = | 2.16 in ⁴ |
| I_x = | 1.07 in ⁴ |
| A = | 1.25 in ² |
| g = | 1.50 lbs/ft |
| M_y = | -1.556 k-ft |
| M_z = | -0.008 k-ft |
| $M_{y \text{ allowable}}$ = | 2.779 k-ft |
| $M_{z \text{ allowable}}$ = | 1.154 k-ft |
| Utilization = | 57% |



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

| | |
|-----------------------------|----------------------|
| Girder Type = | BF0 |
| Aluminum Type = | 6105-T5 |
| F_{ty} = | 35 ksi |
| L_b = | 104.56 in |
| ΦF_{ty} AXIAL = | 31.09 ksi |
| ΦF_{ty} STRONG-AXIS = | 29.00 ksi |
| ΦF_{ty} WEAK-AXIS = | 33.25 ksi |
| S_y = | 1.42 in ³ |
| S_x = | 1.41 in ³ |
| E = | 10100 ksi |
| I_y = | 2.39 in ⁴ |
| I_x = | 2.22 in ⁴ |
| A = | 1.88 in ² |
| g = | 2.26 lbs/ft |
| M_y = | -3.165 k-ft |
| M_z = | 0.000 k-ft |
| P_n = | 1.927 k |
| $M_{y \text{ allowable}}$ = | 3.422 k-ft |
| $M_{z \text{ allowable}}$ = | 3.907 k-ft |
| $P_{n \text{ allowable}}$ = | 58.535 k |
| Utilization = | 96% |



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

| | |
|---------------------------------|----------------------|
| Strut Type = | 55x55 |
| Aluminum Type = | 6105-T5 |
| F_{ty} = | 35 ksi |
| L_b = | 24.80 in |
| $\Phi F_{ty \text{ AXIAL}}$ = | 28.03 ksi |
| $\Phi F_{ty \text{ BENDING}}$ = | 28.22 ksi |
| S_y = | 0.60 in ³ |
| S_x = | 0.60 in ³ |
| E = | 10100 ksi |
| I_y = | 0.67 in ⁴ |
| I_x = | 0.67 in ⁴ |
| A = | 0.98 in ² |
| g = | 1.18 lbs/ft |
| M_y = | 0.000 k-ft |
| M_z = | 0.000 k-ft |
| P_n = | 2.392 k |
| $M_{y \text{ allowable}}$ = | 1.408 k-ft |
| $M_{z \text{ allowable}}$ = | 1.408 k-ft |
| $P_{n \text{ allowable}}$ = | 27.532 k |
| Utilization = | 9% |



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

| | |
|---------------------------------|----------------------|
| Strut Type = | 55x55 |
| Aluminum Type = | 6105-T5 |
| F_{ty} = | 35 ksi |
| L_b = | 98.03 in |
| $\Phi F_{ty \text{ AXIAL}}$ = | 6.11 ksi |
| $\Phi F_{ty \text{ BENDING}}$ = | 28.22 ksi |
| S_y = | 0.60 in ³ |
| S_x = | 0.60 in ³ |
| E = | 10100 ksi |
| I_y = | 0.67 in ⁴ |
| I_x = | 0.67 in ⁴ |
| A = | 0.98 in ² |
| g = | 1.18 lbs/ft |
| M_y = | 0.012 k-ft |
| M_z = | 0.000 k-ft |
| P_n = | 2.731 k |
| $M_{y \text{ allowable}}$ = | 1.408 k-ft |
| $M_{z \text{ allowable}}$ = | 1.408 k-ft |
| $P_{n \text{ allowable}}$ = | 6.000 k |
| Utilization = | 46% |



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

| | |
|---------------------------------|----------------------|
| Strut Type = | 55x55 |
| Aluminum Type = | 6105-T5 |
| F_{ty} = | 35 ksi |
| L_b = | 78.35 in |
| $\Phi F_{ty \text{ AXIAL}}$ = | 8.88 ksi |
| $\Phi F_{ty \text{ BENDING}}$ = | 28.22 ksi |
| S_y = | 0.60 in ³ |
| S_x = | 0.60 in ³ |
| E = | 10100 ksi |
| I_y = | 0.67 in ⁴ |
| I_x = | 0.67 in ⁴ |
| A = | 0.98 in ² |
| g = | 1.18 lbs/ft |
| M_y = | -0.012 k-ft |
| M_z = | 0.000 k-ft |
| P_n = | 3.187 k |
| $M_{y \text{ allowable}}$ = | 1.408 k-ft |
| $M_{z \text{ allowable}}$ = | 1.408 k-ft |
| $P_{n \text{ allowable}}$ = | 8.726 k |
| Utilization = | 37% |



5. FOUNDATION DESIGN CALCULATIONS

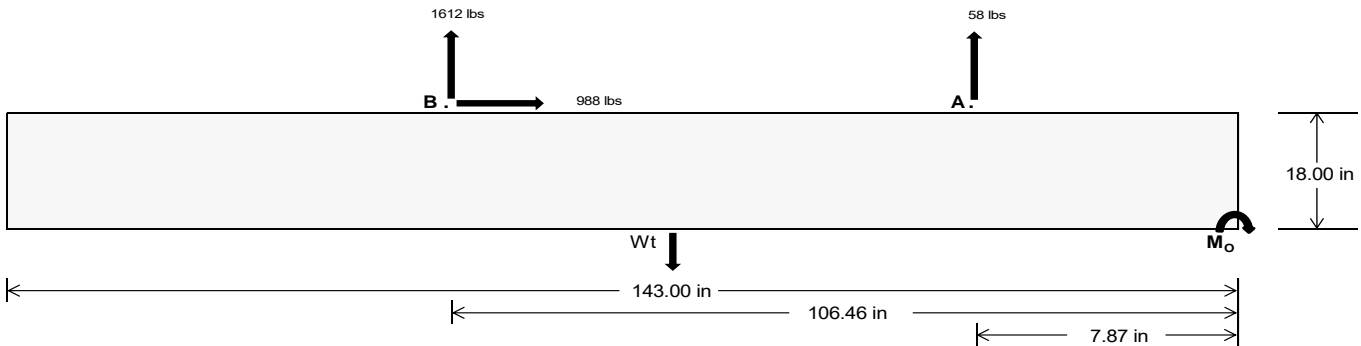
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

| | Maximum | Front | Rear |
|----------------------|---------|----------------|------------------|
| Tensile Load = | | 266.96 | 6994.15 k |
| Compressive Load = | | 3110.08 | 4995.82 k |
| Lateral Load = | | 8.56 | 4281.29 k |
| Moment (Weak Axis) = | | 0.02 | 0.00 k |

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 189808.4$ in-lbs
Resisting Force Required = 2654.66 lbs
S.F. = 1.67
Weight Required = 4424.44 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 988.38 lbs
Friction = 0.4
Weight Required = 2470.95 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 988.38 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$ 35 in 36 in 37 in 38 in
7560 lbs 7776 lbs 7992 lbs 8208 lbs

| ASD LC | 1.0D + 1.0S | | | | 1.0D + 0.6W | | | | 1.0D + 0.75L + 0.45W + 0.75S | | | | 0.6D + 0.6W | | | |
|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|------------------------------|-------------|-------------|-------------|-------------|-------------|-------------|-------------|
| Width | 35 in | 36 in | 37 in | 38 in | 35 in | 36 in | 37 in | 38 in | 35 in | 36 in | 37 in | 38 in | 35 in | 36 in | 37 in | 38 in |
| F_A | 878 lbs | 878 lbs | 878 lbs | 878 lbs | 1282 lbs | 1282 lbs | 1282 lbs | 1282 lbs | 1525 lbs | 1525 lbs | 1525 lbs | 1525 lbs | -116 lbs | -116 lbs | -116 lbs | -116 lbs |
| F_B | 844 lbs | 844 lbs | 844 lbs | 844 lbs | 2157 lbs | 2157 lbs | 2157 lbs | 2157 lbs | 2159 lbs | 2159 lbs | 2159 lbs | 2159 lbs | -3223 lbs | -3223 lbs | -3223 lbs | -3223 lbs |
| F_V | 97 lbs | 97 lbs | 97 lbs | 97 lbs | 1767 lbs | 1767 lbs | 1767 lbs | 1767 lbs | 1388 lbs | 1388 lbs | 1388 lbs | 1388 lbs | -1977 lbs | -1977 lbs | -1977 lbs | -1977 lbs |
| P_{total} | 9281 lbs | 9497 lbs | 9713 lbs | 9929 lbs | 10999 lbs | 11215 lbs | 11431 lbs | 11647 lbs | 11244 lbs | 11460 lbs | 11676 lbs | 11892 lbs | 1197 lbs | 1326 lbs | 1456 lbs | 1586 lbs |
| M | 2343 lbs-ft | 2343 lbs-ft | 2343 lbs-ft | 2343 lbs-ft | 3167 lbs-ft | 3167 lbs-ft | 3167 lbs-ft | 3167 lbs-ft | 3879 lbs-ft | 3879 lbs-ft | 3879 lbs-ft | 3879 lbs-ft | 5809 lbs-ft | 5809 lbs-ft | 5809 lbs-ft | 5809 lbs-ft |
| e | 0.25 ft | 0.25 ft | 0.24 ft | 0.24 ft | 0.29 ft | 0.28 ft | 0.28 ft | 0.27 ft | 0.35 ft | 0.34 ft | 0.33 ft | 0.33 ft | 4.85 ft | 4.38 ft | 3.99 ft | 3.66 ft |
| $L/6$ | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft | 1.99 ft |
| f_{min} | 233.1 psf | 232.7 psf | 232.3 psf | 231.9 psf | 270.6 psf | 269.1 psf | 267.7 psf | 266.4 psf | 267.3 psf | 265.9 psf | 264.6 psf | 263.4 psf | 0.0 psf | 0.0 psf | 0.0 psf | 0.0 psf |
| f_{max} | 301.0 psf | 298.7 psf | 296.5 psf | 294.4 psf | 362.3 psf | 358.3 psf | 354.5 psf | 350.9 psf | 379.7 psf | 375.2 psf | 370.9 psf | 366.9 psf | 247.8 psf | 186.7 psf | 159.9 psf | 145.5 psf |

Maximum Bearing Pressure = 380 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

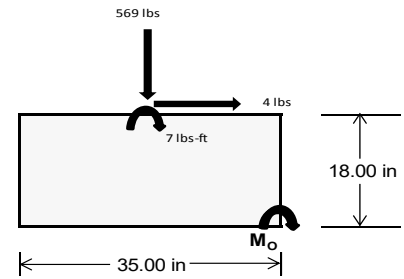
Overturning Check

$M_o = 817.9 \text{ ft-lbs}$
 Resisting Force Required = 560.87 lbs
 S.F. = 1.67
 Weight Required = 934.78 lbs
 Minimum Width = **35 in**
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

| ASD LC | 1.238D + 0.875E | | | 1.1785D + 0.65625E + 0.75S | | | 0.362D + 0.875E | | |
|-------------|-----------------|-----------|-----------|----------------------------|-----------|-----------|-----------------|-----------|----------|
| Width | 35 in | | | 35 in | | | 35 in | | |
| Support | Outer | Inner | Outer | Outer | Inner | Outer | Outer | Inner | Outer |
| F_v | 206 lbs | 462 lbs | 206 lbs | 569 lbs | 1451 lbs | 569 lbs | 60 lbs | 135 lbs | 60 lbs |
| F_v | 1 lbs | 0 lbs | 1 lbs | 4 lbs | 0 lbs | 4 lbs | 0 lbs | 0 lbs | 0 lbs |
| P_{total} | 9565 lbs | 7560 lbs | 9565 lbs | 9478 lbs | 7560 lbs | 9478 lbs | 2797 lbs | 7560 lbs | 2797 lbs |
| M | 4 lbs-ft | 0 lbs-ft | 4 lbs-ft | 12 lbs-ft | 0 lbs-ft | 12 lbs-ft | 0 lbs-ft | 0 lbs-ft | 0 lbs-ft |
| e | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft | 0.00 ft |
| $L/6$ | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft | 0.49 ft |
| f_{min} | 275.0 psf | 217.5 psf | 275.0 psf | 272.0 psf | 217.5 psf | 272.0 psf | 80.4 psf | 217.5 psf | 80.4 psf |
| f_{max} | 275.4 psf | 217.5 psf | 275.4 psf | 273.4 psf | 217.5 psf | 273.4 psf | 80.5 psf | 217.5 psf | 80.5 psf |



Maximum Bearing Pressure = 275 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

| | |
|---------------------------|------------|
| Maximum Uplifting Force = | 1.163 k |
| Allowable Uplift = | 1.214 k |
| Utilization = | <u>96%</u> |



Fastening of Purlins to Girders

| | |
|---------------------------|------------|
| Maximum Uplifting Force = | 2.609 k |
| Allowable Uplift = | 4.357 k |
| Utilization = | <u>60%</u> |



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

| | |
|--------------------------|------------|
| Maximum Axial Load = | 2.392 k |
| M12 Bolt Capacity = | 12.808 k |
| Strut Bearing Capacity = | 7.421 k |
| Utilization = | <u>32%</u> |

Rear Strut

| | |
|--------------------------|------------|
| Maximum Axial Load = | 4.714 k |
| M12 Bolt Capacity = | 12.808 k |
| Strut Bearing Capacity = | 7.421 k |
| Utilization = | <u>64%</u> |

Diagonal Strut

| | |
|---------------------------|------------|
| Maximum Axial Load = | 2.888 k |
| M12 Bolt Shear Capacity = | 12.808 k |
| Strut Bearing Capacity = | 7.421 k |
| Utilization = | <u>39%</u> |

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

| | |
|--|----------------|
| Mean Height, h_{sx} = | 60.93 in |
| Allowable Story Drift for All Other Structures, Δ = { | 0.020 h_{sx} |
| Max Drift, Δ_{MAX} = | 1.219 in |
| | <u>N/A</u> |

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 75 \text{ in}$$

$$J = 0.432$$

$$207.485$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75$$

$$J = 0.432$$

$$131.948$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

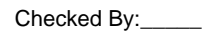
3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMax 72 Cell 2V 30° 160mph 30psf 6.25ft 7-10 Page 3 of 3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

| | Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|----|--------|-----|-----|-----------|----|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 27 | | 14 | max | 37.019 | 1 | 219.331 | 2 | 4.039 | 3 | .011 | 3 | -.003 | 15 | .763 | 3 |
| 28 | | | min | 1.818 | 15 | -366.185 | 3 | -25.933 | 1 | 0 | 15 | -.074 | 1 | -.346 | 2 |
| 29 | | 15 | max | 37.019 | 1 | 91.736 | 2 | 6.213 | 3 | .011 | 3 | -.003 | 12 | .936 | 3 |
| 30 | | | min | 1.818 | 15 | -131.85 | 3 | -1.667 | 10 | 0 | 15 | -.081 | 1 | -.454 | 2 |
| 31 | | 16 | max | 37.019 | 1 | 102.485 | 3 | 35.596 | 1 | .011 | 3 | 0 | 3 | .946 | 3 |
| 32 | | | min | 1.818 | 15 | -35.86 | 2 | 1.75 | 15 | 0 | 15 | -.067 | 1 | -.473 | 2 |
| 33 | | 17 | max | 37.019 | 1 | 336.82 | 3 | 66.361 | 1 | .011 | 3 | .007 | 3 | .793 | 3 |
| 34 | | | min | 1.818 | 15 | -163.456 | 2 | 3.176 | 15 | 0 | 15 | -.032 | 1 | -.404 | 2 |
| 35 | | 18 | max | 37.019 | 1 | 571.155 | 3 | 97.125 | 1 | .011 | 3 | .025 | 1 | .478 | 3 |
| 36 | | | min | 1.818 | 15 | -291.052 | 2 | 4.602 | 15 | 0 | 15 | -.002 | 10 | -.246 | 2 |
| 37 | | 19 | max | 37.019 | 1 | 805.491 | 3 | 127.89 | 1 | .011 | 3 | .103 | 1 | 0 | 2 |
| 38 | | | min | 1.818 | 15 | -418.647 | 2 | 6.028 | 15 | 0 | 15 | .005 | 15 | 0 | 3 |
| 39 | M14 | 1 | max | 27.25 | 1 | 524.837 | 2 | -6.336 | 15 | .016 | 3 | .13 | 1 | 0 | 2 |
| 40 | | | min | 1.324 | 15 | -683.757 | 3 | -134.353 | 1 | -.016 | 2 | .006 | 15 | 0 | 3 |
| 41 | | 2 | max | 27.25 | 1 | 397.241 | 2 | -4.91 | 15 | .016 | 3 | .047 | 1 | .413 | 3 |
| 42 | | | min | 1.324 | 15 | -504.369 | 3 | -103.589 | 1 | -.016 | 2 | 0 | 10 | -.32 | 2 |
| 43 | | 3 | max | 27.25 | 1 | 269.646 | 2 | -3.483 | 15 | .016 | 3 | .009 | 3 | .701 | 3 |
| 44 | | | min | 1.324 | 15 | -324.98 | 3 | -72.824 | 1 | -.016 | 2 | -.014 | 1 | -.552 | 2 |
| 45 | | 4 | max | 27.25 | 1 | 142.05 | 2 | -2.057 | 15 | .016 | 3 | .002 | 3 | .864 | 3 |
| 46 | | | min | 1.324 | 15 | -145.591 | 3 | -42.059 | 1 | -.016 | 2 | -.054 | 1 | -.695 | 2 |
| 47 | | 5 | max | 27.25 | 1 | 33.797 | 3 | 1.024 | 10 | .016 | 3 | -.002 | 12 | .903 | 3 |
| 48 | | | min | 1.324 | 15 | -.507 | 9 | -11.295 | 1 | -.016 | 2 | -.073 | 1 | -.749 | 2 |
| 49 | | 6 | max | 27.25 | 1 | 213.186 | 3 | 19.47 | 1 | .016 | 3 | -.003 | 15 | .817 | 3 |
| 50 | | | min | 1.324 | 15 | -113.141 | 2 | -4.76 | 3 | -.016 | 2 | -.07 | 1 | -.715 | 2 |
| 51 | | 7 | max | 27.25 | 1 | 392.575 | 3 | 50.235 | 1 | .016 | 3 | -.002 | 15 | .607 | 3 |
| 52 | | | min | 1.324 | 15 | -240.737 | 2 | -2.586 | 3 | -.016 | 2 | -.046 | 1 | -.592 | 2 |
| 53 | | 8 | max | 27.25 | 1 | 571.963 | 3 | 80.999 | 1 | .016 | 3 | .006 | 2 | .272 | 3 |
| 54 | | | min | 1.324 | 15 | -368.333 | 2 | -.412 | 3 | -.016 | 2 | -.011 | 3 | -.38 | 2 |
| 55 | | 9 | max | 27.25 | 1 | 751.352 | 3 | 111.764 | 1 | .016 | 3 | .067 | 1 | .003 | 9 |
| 56 | | | min | 1.324 | 15 | -495.929 | 2 | 1.565 | 12 | -.016 | 2 | -.01 | 3 | -.188 | 3 |
| 57 | | 10 | max | 27.25 | 1 | 930.741 | 3 | 142.529 | 1 | .016 | 2 | .155 | 1 | .308 | 2 |
| 58 | | | min | 1.324 | 15 | -623.524 | 2 | -20.723 | 10 | -.016 | 3 | -.008 | 3 | -.772 | 3 |
| 59 | | 11 | max | 27.25 | 1 | 495.929 | 2 | -1.565 | 12 | .016 | 2 | .067 | 1 | .003 | 9 |
| 60 | | | min | 1.324 | 15 | -751.352 | 3 | -111.764 | 1 | -.016 | 3 | -.01 | 3 | -.188 | 3 |
| 61 | | 12 | max | 27.25 | 1 | 368.333 | 2 | .412 | 3 | .016 | 2 | .006 | 2 | .272 | 3 |
| 62 | | | min | 1.324 | 15 | -571.963 | 3 | -80.999 | 1 | -.016 | 3 | -.011 | 3 | -.38 | 2 |
| 63 | | 13 | max | 27.25 | 1 | 240.737 | 2 | 2.586 | 3 | .016 | 2 | -.002 | 15 | .607 | 3 |
| 64 | | | min | 1.324 | 15 | -392.575 | 3 | -50.235 | 1 | -.016 | 3 | -.046 | 1 | -.592 | 2 |
| 65 | | 14 | max | 27.25 | 1 | 113.141 | 2 | 4.76 | 3 | .016 | 2 | -.003 | 15 | .817 | 3 |
| 66 | | | min | 1.324 | 15 | -213.186 | 3 | -19.47 | 1 | -.016 | 3 | -.07 | 1 | -.715 | 2 |
| 67 | | 15 | max | 27.25 | 1 | .507 | 9 | 11.295 | 1 | .016 | 2 | -.002 | 12 | .903 | 3 |
| 68 | | | min | 1.324 | 15 | -33.797 | 3 | -1.024 | 10 | -.016 | 3 | -.073 | 1 | -.749 | 2 |
| 69 | | 16 | max | 27.25 | 1 | 145.591 | 3 | 42.059 | 1 | .016 | 2 | .002 | 3 | .864 | 3 |
| 70 | | | min | 1.324 | 15 | -142.05 | 2 | 2.057 | 15 | -.016 | 3 | -.054 | 1 | -.695 | 2 |
| 71 | | 17 | max | 27.25 | 1 | 324.98 | 3 | 72.824 | 1 | .016 | 2 | .009 | 3 | .701 | 3 |
| 72 | | | min | 1.324 | 15 | -269.646 | 2 | 3.483 | 15 | -.016 | 3 | -.014 | 1 | -.552 | 2 |
| 73 | | 18 | max | 27.25 | 1 | 504.369 | 3 | 103.589 | 1 | .016 | 2 | .047 | 1 | .413 | 3 |
| 74 | | | min | 1.324 | 15 | -397.241 | 2 | 4.91 | 15 | -.016 | 3 | 0 | 10 | -.32 | 2 |
| 75 | | 19 | max | 27.25 | 1 | 683.757 | 3 | 134.353 | 1 | .016 | 2 | .13 | 1 | 0 | 2 |
| 76 | | | min | 1.324 | 15 | -524.837 | 2 | 6.336 | 15 | -.016 | 3 | .006 | 15 | 0 | 3 |
| 77 | M15 | 1 | max | -1.39 | 15 | 741.519 | 2 | -6.332 | 15 | .017 | 2 | .13 | 1 | 0 | 2 |
| 78 | | | min | -28.227 | 1 | -406.916 | 3 | -134.415 | 1 | -.013 | 3 | .006 | 15 | 0 | 3 |
| 79 | | 2 | max | -1.39 | 15 | 549.819 | 2 | -4.906 | 15 | .017 | 2 | .047 | 1 | .249 | 3 |
| 80 | | | min | -28.227 | 1 | -309.947 | 3 | -103.651 | 1 | -.013 | 3 | 0 | 10 | -.448 | 2 |
| 81 | | 3 | max | -1.39 | 15 | 358.119 | 2 | -3.479 | 15 | .017 | 2 | .008 | 3 | .43 | 3 |
| 82 | | | min | -28.227 | 1 | -212.977 | 3 | -72.886 | 1 | -.013 | 3 | -.014 | 1 | -.764 | 2 |
| 83 | | 4 | max | -1.39 | 15 | 166.419 | 2 | -2.053 | 15 | .017 | 2 | .002 | 3 | .545 | 3 |



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 160mph 30psf 6.25ft 7-10PaSe 21



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Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

| | Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|-----|--------|-----|-----|-----------|----|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 141 | | 14 | max | -2.059 | 15 | 315.925 | 2 | 2.399 | 3 | .011 | 3 | -.003 | 15 | .244 | 3 |
| 142 | | | min | -42.288 | 1 | -172.042 | 3 | -25.17 | 1 | -.003 | 1 | -.074 | 1 | -.567 | 2 |
| 143 | | 15 | max | -2.059 | 15 | 124.225 | 2 | 5.594 | 1 | .011 | 3 | -.003 | 12 | .33 | 3 |
| 144 | | | min | -42.288 | 1 | -75.073 | 3 | -1.097 | 10 | -.003 | 1 | -.081 | 1 | -.72 | 2 |
| 145 | | 16 | max | -2.059 | 15 | 21.897 | 3 | 36.359 | 1 | .011 | 3 | -.001 | 12 | .349 | 3 |
| 146 | | | min | -42.288 | 1 | -67.475 | 2 | 1.764 | 15 | -.003 | 1 | -.066 | 1 | -.74 | 2 |
| 147 | | 17 | max | -2.059 | 15 | 118.866 | 3 | 67.124 | 1 | .011 | 3 | .004 | 3 | .3 | 3 |
| 148 | | | min | -42.288 | 1 | -259.175 | 2 | 3.191 | 15 | -.003 | 1 | -.03 | 1 | -.626 | 2 |
| 149 | | 18 | max | -2.059 | 15 | 215.835 | 3 | 97.888 | 1 | .011 | 3 | .027 | 1 | .184 | 3 |
| 150 | | | min | -42.288 | 1 | -450.875 | 2 | 4.617 | 15 | -.003 | 1 | 0 | 10 | -.38 | 2 |
| 151 | | 19 | max | -2.059 | 15 | 312.804 | 3 | 128.653 | 1 | .011 | 3 | .105 | 1 | 0 | 2 |
| 152 | | | min | -42.288 | 1 | -642.574 | 2 | 6.043 | 15 | -.003 | 1 | .005 | 15 | 0 | 3 |
| 153 | M2 | 1 | max | 1103.186 | 2 | 2.025 | 4 | .203 | 1 | 0 | 3 | 0 | 3 | 0 | 1 |
| 154 | | | min | -1581.815 | 3 | .476 | 15 | .01 | 15 | 0 | 1 | 0 | 2 | 0 | 1 |
| 155 | | 2 | max | 1103.715 | 2 | 1.954 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 156 | | | min | -1581.418 | 3 | .459 | 15 | .01 | 15 | 0 | 1 | 0 | 10 | 0 | 4 |
| 157 | | 3 | max | 1104.244 | 2 | 1.883 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 158 | | | min | -1581.021 | 3 | .443 | 15 | .01 | 15 | 0 | 1 | 0 | 10 | -.001 | 4 |
| 159 | | 4 | max | 1104.774 | 2 | 1.812 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 160 | | | min | -1580.624 | 3 | .426 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.002 | 4 |
| 161 | | 5 | max | 1105.303 | 2 | 1.741 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 162 | | | min | -1580.227 | 3 | .409 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.003 | 4 |
| 163 | | 6 | max | 1105.832 | 2 | 1.67 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 164 | | | min | -1579.83 | 3 | .393 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.003 | 4 |
| 165 | | 7 | max | 1106.362 | 2 | 1.599 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | 0 | 15 |
| 166 | | | min | -1579.433 | 3 | .376 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.004 | 4 |
| 167 | | 8 | max | 1106.891 | 2 | 1.528 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.001 | 15 |
| 168 | | | min | -1579.036 | 3 | .359 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.004 | 4 |
| 169 | | 9 | max | 1107.42 | 2 | 1.457 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.001 | 15 |
| 170 | | | min | -1578.639 | 3 | .343 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.005 | 4 |
| 171 | | 10 | max | 1107.949 | 2 | 1.386 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.001 | 15 |
| 172 | | | min | -1578.242 | 3 | .326 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.006 | 4 |
| 173 | | 11 | max | 1108.479 | 2 | 1.315 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.001 | 15 |
| 174 | | | min | -1577.845 | 3 | .309 | 15 | .01 | 15 | 0 | 1 | 0 | 15 | -.006 | 4 |
| 175 | | 12 | max | 1109.008 | 2 | 1.244 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.002 | 15 |
| 176 | | | min | -1577.448 | 3 | .29 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.006 | 4 |
| 177 | | 13 | max | 1109.537 | 2 | 1.173 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.002 | 15 |
| 178 | | | min | -1577.052 | 3 | .262 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.007 | 4 |
| 179 | | 14 | max | 1110.067 | 2 | 1.102 | 4 | .203 | 1 | 0 | 3 | 0 | 1 | -.002 | 15 |
| 180 | | | min | -1576.655 | 3 | .235 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.007 | 4 |
| 181 | | 15 | max | 1110.596 | 2 | 1.041 | 2 | .203 | 1 | 0 | 3 | .001 | 1 | -.002 | 15 |
| 182 | | | min | -1576.258 | 3 | .207 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.008 | 4 |
| 183 | | 16 | max | 1111.125 | 2 | .986 | 2 | .203 | 1 | 0 | 3 | .001 | 1 | -.002 | 15 |
| 184 | | | min | -1575.861 | 3 | .179 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.008 | 4 |
| 185 | | 17 | max | 1111.654 | 2 | .93 | 2 | .203 | 1 | 0 | 3 | .001 | 1 | -.002 | 15 |
| 186 | | | min | -1575.464 | 3 | .152 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.008 | 4 |
| 187 | | 18 | max | 1112.184 | 2 | .875 | 2 | .203 | 1 | 0 | 3 | .001 | 1 | -.002 | 15 |
| 188 | | | min | -1575.067 | 3 | .124 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.009 | 4 |
| 189 | | 19 | max | 1112.713 | 2 | .82 | 2 | .203 | 1 | 0 | 3 | .001 | 1 | -.002 | 15 |
| 190 | | | min | -1574.67 | 3 | .096 | 12 | .01 | 15 | 0 | 1 | 0 | 15 | -.009 | 4 |
| 191 | M3 | 1 | max | 897.74 | 2 | 8.877 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | .009 | 4 |
| 192 | | | min | -1021.088 | 3 | 2.087 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | .002 | 15 |
| 193 | | 2 | max | 897.569 | 2 | 8.008 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | .005 | 2 |
| 194 | | | min | -1021.215 | 3 | 1.882 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | 0 | 12 |
| 195 | | 3 | max | 897.399 | 2 | 7.139 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | .002 | 2 |
| 196 | | | min | -1021.343 | 3 | 1.678 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.001 | 3 |
| 197 | | 4 | max | 897.229 | 2 | 6.27 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | 0 | 2 |



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Envelope Member Section Forces (Continued)

| | Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|-----|--------|-----|-----|-----------|----|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 198 | | | min | -1021.471 | 3 | 1.474 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.003 | 3 |
| 199 | | 5 | max | 897.058 | 2 | 5.401 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.001 | 15 |
| 200 | | | min | -1021.599 | 3 | 1.27 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.004 | 4 |
| 201 | | 6 | max | 896.888 | 2 | 4.532 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.002 | 15 |
| 202 | | | min | -1021.726 | 3 | 1.065 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.007 | 4 |
| 203 | | 7 | max | 896.717 | 2 | 3.663 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.002 | 15 |
| 204 | | | min | -1021.854 | 3 | .861 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.009 | 4 |
| 205 | | 8 | max | 896.547 | 2 | 2.794 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.002 | 15 |
| 206 | | | min | -1021.982 | 3 | .657 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.01 | 4 |
| 207 | | 9 | max | 896.377 | 2 | 1.925 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.003 | 15 |
| 208 | | | min | -1022.11 | 3 | .453 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.011 | 4 |
| 209 | | 10 | max | 896.206 | 2 | 1.057 | 4 | .178 | 1 | 0 | 5 | 0 | 1 | -.003 | 15 |
| 210 | | | min | -1022.237 | 3 | .248 | 15 | .009 | 15 | 0 | 1 | 0 | 15 | -.012 | 4 |
| 211 | | 11 | max | 896.036 | 2 | .327 | 2 | .178 | 1 | 0 | 5 | 0 | 1 | -.003 | 15 |
| 212 | | | min | -1022.365 | 3 | -.138 | 3 | .009 | 15 | 0 | 1 | 0 | 15 | -.012 | 4 |
| 213 | | 12 | max | 895.866 | 2 | -.16 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.003 | 15 |
| 214 | | | min | -1022.493 | 3 | -.681 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.012 | 4 |
| 215 | | 13 | max | 895.695 | 2 | -.364 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.003 | 15 |
| 216 | | | min | -1022.621 | 3 | -1.55 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.012 | 4 |
| 217 | | 14 | max | 895.525 | 2 | -.569 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.003 | 15 |
| 218 | | | min | -1022.748 | 3 | -2.419 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.011 | 4 |
| 219 | | 15 | max | 895.355 | 2 | -.773 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.002 | 15 |
| 220 | | | min | -1022.876 | 3 | -3.288 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.009 | 4 |
| 221 | | 16 | max | 895.184 | 2 | -.977 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.002 | 15 |
| 222 | | | min | -1023.004 | 3 | -4.157 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.008 | 4 |
| 223 | | 17 | max | 895.014 | 2 | -1.181 | 15 | .178 | 1 | 0 | 5 | .001 | 1 | -.001 | 15 |
| 224 | | | min | -1023.132 | 3 | -5.026 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.006 | 4 |
| 225 | | 18 | max | 894.844 | 2 | -1.386 | 15 | .178 | 1 | 0 | 5 | .002 | 1 | 0 | 15 |
| 226 | | | min | -1023.26 | 3 | -5.895 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | -.003 | 4 |
| 227 | | 19 | max | 894.673 | 2 | -1.59 | 15 | .178 | 1 | 0 | 5 | .002 | 1 | 0 | 1 |
| 228 | | | min | -1023.387 | 3 | -6.764 | 4 | .009 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 229 | M4 | 1 | max | 862.11 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | .001 | 1 | 0 | 1 |
| 230 | | | min | -33.779 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | 0 | 15 | 0 | 1 |
| 231 | | 2 | max | 862.28 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 1 | 0 | 1 |
| 232 | | | min | -33.651 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | 0 | 15 | 0 | 1 |
| 233 | | 3 | max | 862.451 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 234 | | | min | -33.523 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| 235 | | 4 | max | 862.621 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 236 | | | min | -33.396 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.001 | 1 | 0 | 1 |
| 237 | | 5 | max | 862.791 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 238 | | | min | -33.268 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.002 | 1 | 0 | 1 |
| 239 | | 6 | max | 862.962 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 240 | | | min | -33.14 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.003 | 1 | 0 | 1 |
| 241 | | 7 | max | 863.132 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 242 | | | min | -33.012 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.003 | 1 | 0 | 1 |
| 243 | | 8 | max | 863.302 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 244 | | | min | -32.884 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.004 | 1 | 0 | 1 |
| 245 | | 9 | max | 863.473 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 246 | | | min | -32.757 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.005 | 1 | 0 | 1 |
| 247 | | 10 | max | 863.643 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 248 | | | min | -32.629 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.006 | 1 | 0 | 1 |
| 249 | | 11 | max | 863.813 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 250 | | | min | -32.501 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.006 | 1 | 0 | 1 |
| 251 | | 12 | max | 863.984 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 252 | | | min | -32.373 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.007 | 1 | 0 | 1 |
| 253 | | 13 | max | 864.154 | 1 | 0 | 1 | -.326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 254 | | | min | -32.246 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.008 | 1 | 0 | 1 |



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Envelope Member Section Forces (Continued)

| Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|--------|-----|-----|-----------|----------|-------------|-------|-------------|----|--------------|----|-------------|----|-------------|----|
| 255 | 14 | max | 864.324 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 256 | | min | -32.118 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.009 | 1 | 0 | 1 |
| 257 | 15 | max | 864.495 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 258 | | min | -31.99 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.01 | 1 | 0 | 1 |
| 259 | 16 | max | 864.665 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 260 | | min | -31.862 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.01 | 1 | 0 | 1 |
| 261 | 17 | max | 864.835 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 262 | | min | -31.735 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.011 | 1 | 0 | 1 |
| 263 | 18 | max | 865.006 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 264 | | min | -31.607 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.012 | 1 | 0 | 1 |
| 265 | 19 | max | 865.176 | 1 | 0 | 1 | -326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 266 | | min | -31.479 | 3 | 0 | 1 | -6.745 | 1 | 0 | 1 | -.013 | 1 | 0 | 1 |
| 267 | M6 | 1 | max | 3177.061 | 2 | 2.29 | 2 | 0 | 1 | 0 | 0 | 1 | 0 | 1 |
| 268 | | min | -4713.517 | 3 | .228 | 12 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| 269 | 2 | max | 3177.59 | 2 | 2.235 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 12 |
| 270 | | min | -4713.12 | 3 | .201 | 12 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 2 |
| 271 | 3 | max | 3178.119 | 2 | 2.179 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 12 |
| 272 | | min | -4712.723 | 3 | .166 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.002 | 2 |
| 273 | 4 | max | 3178.649 | 2 | 2.124 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 12 |
| 274 | | min | -4712.326 | 3 | .125 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.002 | 2 |
| 275 | 5 | max | 3179.178 | 2 | 2.069 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 276 | | min | -4711.929 | 3 | .083 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.003 | 2 |
| 277 | 6 | max | 3179.707 | 2 | 2.013 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 278 | | min | -4711.532 | 3 | .042 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.004 | 2 |
| 279 | 7 | max | 3180.237 | 2 | 1.958 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 280 | | min | -4711.135 | 3 | 0 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.005 | 2 |
| 281 | 8 | max | 3180.766 | 2 | 1.903 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 282 | | min | -4710.738 | 3 | -.041 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.005 | 2 |
| 283 | 9 | max | 3181.295 | 2 | 1.847 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 284 | | min | -4710.341 | 3 | -.083 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.006 | 2 |
| 285 | 10 | max | 3181.824 | 2 | 1.792 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 286 | | min | -4709.944 | 3 | -.124 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.007 | 2 |
| 287 | 11 | max | 3182.354 | 2 | 1.737 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 288 | | min | -4709.547 | 3 | -.166 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.007 | 2 |
| 289 | 12 | max | 3182.883 | 2 | 1.681 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 290 | | min | -4709.15 | 3 | -.207 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.008 | 2 |
| 291 | 13 | max | 3183.412 | 2 | 1.626 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 292 | | min | -4708.753 | 3 | -.249 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.008 | 2 |
| 293 | 14 | max | 3183.942 | 2 | 1.571 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 294 | | min | -4708.356 | 3 | -.29 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.009 | 2 |
| 295 | 15 | max | 3184.471 | 2 | 1.515 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 296 | | min | -4707.959 | 3 | -.332 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.01 | 2 |
| 297 | 16 | max | 3185 | 2 | 1.46 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 298 | | min | -4707.562 | 3 | -.373 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.01 | 2 |
| 299 | 17 | max | 3185.529 | 2 | 1.405 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 300 | | min | -4707.165 | 3 | -.415 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.011 | 2 |
| 301 | 18 | max | 3186.059 | 2 | 1.349 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 302 | | min | -4706.768 | 3 | -.456 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.011 | 2 |
| 303 | 19 | max | 3186.588 | 2 | 1.294 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 304 | | min | -4706.371 | 3 | -.498 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -.012 | 2 |
| 305 | M7 | 1 | max | 2731.012 | 2 | 8.894 | 4 | 0 | 1 | 0 | 0 | 1 | .012 | 2 |
| 306 | | min | -2886.101 | 3 | 2.089 | 15 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 3 |
| 307 | 2 | max | 2730.842 | 2 | 8.025 | 4 | 0 | 1 | 0 | 1 | 0 | 1 | .008 | 2 |
| 308 | | min | -2886.229 | 3 | 1.885 | 15 | 0 | 1 | 0 | 1 | 0 | 1 | -.003 | 3 |
| 309 | 3 | max | 2730.672 | 2 | 7.156 | 4 | 0 | 1 | 0 | 1 | 0 | 1 | .005 | 2 |
| 310 | | min | -2886.357 | 3 | 1.681 | 15 | 0 | 1 | 0 | 1 | 0 | 1 | -.005 | 3 |
| 311 | 4 | max | 2730.501 | 2 | 6.287 | 4 | 0 | 1 | 0 | 1 | 0 | 1 | .002 | 2 |







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

| Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|--------|-----|-----|-----------|--------|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 426 | | min | -1021.471 | 3 | 1.474 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.003 | 3 |
| 427 | 5 | max | 897.058 | 2 | 5.401 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.001 | 15 |
| 428 | | min | -1021.599 | 3 | 1.27 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.004 | 4 |
| 429 | 6 | max | 896.888 | 2 | 4.532 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.002 | 15 |
| 430 | | min | -1021.726 | 3 | 1.065 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.007 | 4 |
| 431 | 7 | max | 896.717 | 2 | 3.663 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.002 | 15 |
| 432 | | min | -1021.854 | 3 | .861 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.009 | 4 |
| 433 | 8 | max | 896.547 | 2 | 2.794 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.002 | 15 |
| 434 | | min | -1021.982 | 3 | .657 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.01 | 4 |
| 435 | 9 | max | 896.377 | 2 | 1.925 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 436 | | min | -1022.11 | 3 | .453 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.011 | 4 |
| 437 | 10 | max | 896.206 | 2 | 1.057 | 4 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 438 | | min | -1022.237 | 3 | .248 | 15 | -.178 | 1 | 0 | 5 | 0 | 1 | -.012 | 4 |
| 439 | 11 | max | 896.036 | 2 | .327 | 2 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 440 | | min | -1022.365 | 3 | -.138 | 3 | -.178 | 1 | 0 | 5 | 0 | 1 | -.012 | 4 |
| 441 | 12 | max | 895.866 | 2 | -.16 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 442 | | min | -1022.493 | 3 | -.681 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.012 | 4 |
| 443 | 13 | max | 895.695 | 2 | -.364 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 444 | | min | -1022.621 | 3 | -1.55 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.012 | 4 |
| 445 | 14 | max | 895.525 | 2 | -.569 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.003 | 15 |
| 446 | | min | -1022.748 | 3 | -2.419 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.011 | 4 |
| 447 | 15 | max | 895.355 | 2 | -.773 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.002 | 15 |
| 448 | | min | -1022.876 | 3 | -3.288 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.009 | 4 |
| 449 | 16 | max | 895.184 | 2 | -.977 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.002 | 15 |
| 450 | | min | -1023.004 | 3 | -4.157 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.008 | 4 |
| 451 | 17 | max | 895.014 | 2 | -1.181 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | -.001 | 15 |
| 452 | | min | -1023.132 | 3 | -5.026 | 4 | -.178 | 1 | 0 | 5 | -.001 | 1 | -.006 | 4 |
| 453 | 18 | max | 894.844 | 2 | -1.386 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | 0 | 15 |
| 454 | | min | -1023.26 | 3 | -5.895 | 4 | -.178 | 1 | 0 | 5 | -.002 | 1 | -.003 | 4 |
| 455 | 19 | max | 894.673 | 2 | -1.59 | 15 | -.009 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 456 | | min | -1023.387 | 3 | -6.764 | 4 | -.178 | 1 | 0 | 5 | -.002 | 1 | 0 | 1 |
| 457 | M12 | 1 | max | 862.11 | 1 | 0 | 6.745 | 1 | 0 | 1 | 0 | 15 | 0 | 1 |
| 458 | | min | -33.779 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | -.001 | 1 | 0 | 1 |
| 459 | 2 | max | 862.28 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | 0 | 15 | 0 | 1 |
| 460 | | min | -33.651 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 1 | 0 | 1 |
| 461 | 3 | max | 862.451 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | 0 | 1 | 0 | 1 |
| 462 | | min | -33.523 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 463 | 4 | max | 862.621 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .001 | 1 | 0 | 1 |
| 464 | | min | -33.396 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 465 | 5 | max | 862.791 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .002 | 1 | 0 | 1 |
| 466 | | min | -33.268 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 467 | 6 | max | 862.962 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .003 | 1 | 0 | 1 |
| 468 | | min | -33.14 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 469 | 7 | max | 863.132 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .003 | 1 | 0 | 1 |
| 470 | | min | -33.012 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 471 | 8 | max | 863.302 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .004 | 1 | 0 | 1 |
| 472 | | min | -32.884 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 473 | 9 | max | 863.473 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .005 | 1 | 0 | 1 |
| 474 | | min | -32.757 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 475 | 10 | max | 863.643 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .006 | 1 | 0 | 1 |
| 476 | | min | -32.629 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 477 | 11 | max | 863.813 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .006 | 1 | 0 | 1 |
| 478 | | min | -32.501 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 479 | 12 | max | 863.984 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .007 | 1 | 0 | 1 |
| 480 | | min | -32.373 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 481 | 13 | max | 864.154 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .008 | 1 | 0 | 1 |
| 482 | | min | -32.246 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |



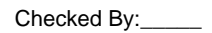
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

| | Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|-----|--------|-----|-----|-----------|----|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 483 | | 14 | max | 864.324 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .009 | 1 | 0 | 1 |
| 484 | | | min | -32.118 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 485 | | 15 | max | 864.495 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .01 | 1 | 0 | 1 |
| 486 | | | min | -31.99 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 487 | | 16 | max | 864.665 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .01 | 1 | 0 | 1 |
| 488 | | | min | -31.862 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 489 | | 17 | max | 864.835 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .011 | 1 | 0 | 1 |
| 490 | | | min | -31.735 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 491 | | 18 | max | 865.006 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .012 | 1 | 0 | 1 |
| 492 | | | min | -31.607 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 493 | | 19 | max | 865.176 | 1 | 0 | 1 | 6.745 | 1 | 0 | 1 | .013 | 1 | 0 | 1 |
| 494 | | | min | -31.479 | 3 | 0 | 1 | .326 | 15 | 0 | 1 | 0 | 15 | 0 | 1 |
| 495 | M1 | 1 | max | 127.894 | 1 | 805.375 | 3 | -1.818 | 15 | 0 | 2 | .103 | 1 | 0 | 15 |
| 496 | | | min | 6.028 | 15 | -417.806 | 2 | -36.975 | 1 | 0 | 3 | .005 | 15 | -.011 | 3 |
| 497 | | 2 | max | 128.737 | 1 | 804.28 | 3 | -1.818 | 15 | 0 | 2 | .08 | 1 | .25 | 2 |
| 498 | | | min | 6.283 | 15 | -419.265 | 2 | -36.975 | 1 | 0 | 3 | .004 | 15 | -.51 | 3 |
| 499 | | 3 | max | 660.743 | 3 | 580.087 | 2 | -1.808 | 15 | 0 | 3 | .057 | 1 | .499 | 2 |
| 500 | | | min | -395.961 | 2 | -649.32 | 3 | -36.843 | 1 | 0 | 2 | .003 | 15 | -.993 | 3 |
| 501 | | 4 | max | 661.375 | 3 | 578.628 | 2 | -1.808 | 15 | 0 | 3 | .034 | 1 | .14 | 2 |
| 502 | | | min | -395.118 | 2 | -650.414 | 3 | -36.843 | 1 | 0 | 2 | .002 | 15 | -.59 | 3 |
| 503 | | 5 | max | 662.006 | 3 | 577.169 | 2 | -1.808 | 15 | 0 | 3 | .011 | 1 | -.005 | 15 |
| 504 | | | min | -394.276 | 2 | -651.509 | 3 | -36.843 | 1 | 0 | 2 | 0 | 15 | -.219 | 2 |
| 505 | | 6 | max | 662.638 | 3 | 575.71 | 2 | -1.808 | 15 | 0 | 3 | 0 | 15 | .219 | 3 |
| 506 | | | min | -393.434 | 2 | -652.603 | 3 | -36.843 | 1 | 0 | 2 | -.012 | 1 | -.577 | 2 |
| 507 | | 7 | max | 663.27 | 3 | 574.251 | 2 | -1.808 | 15 | 0 | 3 | -.002 | 15 | .624 | 3 |
| 508 | | | min | -392.591 | 2 | -653.697 | 3 | -36.843 | 1 | 0 | 2 | -.034 | 1 | -.933 | 2 |
| 509 | | 8 | max | 663.902 | 3 | 572.792 | 2 | -1.808 | 15 | 0 | 3 | -.003 | 15 | 1.03 | 3 |
| 510 | | | min | -391.749 | 2 | -654.792 | 3 | -36.843 | 1 | 0 | 2 | -.057 | 1 | -1.289 | 2 |
| 511 | | 9 | max | 680.165 | 3 | 53.174 | 2 | -3.132 | 15 | 0 | 9 | .039 | 1 | 1.195 | 3 |
| 512 | | | min | -338.38 | 2 | .445 | 15 | -64.123 | 1 | 0 | 3 | .002 | 15 | -1.468 | 2 |
| 513 | | 10 | max | 680.796 | 3 | 51.715 | 2 | -3.132 | 15 | 0 | 9 | 0 | 10 | 1.174 | 3 |
| 514 | | | min | -337.538 | 2 | .005 | 15 | -64.123 | 1 | 0 | 3 | 0 | 1 | -1.501 | 2 |
| 515 | | 11 | max | 681.428 | 3 | 50.256 | 2 | -3.132 | 15 | 0 | 9 | -.002 | 15 | 1.152 | 3 |
| 516 | | | min | -336.695 | 2 | -1.793 | 4 | -64.123 | 1 | 0 | 3 | -.04 | 1 | -1.532 | 2 |
| 517 | | 12 | max | 697.163 | 3 | 448.536 | 3 | -1.742 | 15 | 0 | 2 | .057 | 1 | 1.015 | 3 |
| 518 | | | min | -283.078 | 2 | -688.095 | 2 | -35.9 | 1 | 0 | 3 | .003 | 15 | -1.362 | 2 |
| 519 | | 13 | max | 697.795 | 3 | 447.441 | 3 | -1.742 | 15 | 0 | 2 | .034 | 1 | .737 | 3 |
| 520 | | | min | -282.236 | 2 | -689.554 | 2 | -35.9 | 1 | 0 | 3 | .002 | 15 | -.935 | 2 |
| 521 | | 14 | max | 698.427 | 3 | 446.347 | 3 | -1.742 | 15 | 0 | 2 | .012 | 1 | .46 | 3 |
| 522 | | | min | -281.393 | 2 | -691.013 | 2 | -35.9 | 1 | 0 | 3 | 0 | 15 | -.507 | 2 |
| 523 | | 15 | max | 699.059 | 3 | 445.253 | 3 | -1.742 | 15 | 0 | 2 | 0 | 15 | .183 | 3 |
| 524 | | | min | -280.551 | 2 | -692.472 | 2 | -35.9 | 1 | 0 | 3 | -.01 | 1 | -.086 | 1 |
| 525 | | 16 | max | 699.691 | 3 | 444.158 | 3 | -1.742 | 15 | 0 | 2 | -.002 | 15 | .353 | 2 |
| 526 | | | min | -279.709 | 2 | -693.931 | 2 | -35.9 | 1 | 0 | 3 | -.033 | 1 | -.093 | 3 |
| 527 | | 17 | max | 700.322 | 3 | 443.064 | 3 | -1.742 | 15 | 0 | 2 | -.003 | 15 | .784 | 2 |
| 528 | | | min | -278.866 | 2 | -695.39 | 2 | -35.9 | 1 | 0 | 3 | -.055 | 1 | -.368 | 3 |
| 529 | | 18 | max | -6.297 | 15 | 644.783 | 2 | -2.059 | 15 | 0 | 3 | -.004 | 15 | .398 | 2 |
| 530 | | | min | -129.492 | 1 | -311.881 | 3 | -42.33 | 1 | 0 | 2 | -.079 | 1 | -.183 | 3 |
| 531 | | 19 | max | -6.043 | 15 | 643.324 | 2 | -2.059 | 15 | 0 | 3 | -.005 | 15 | .011 | 3 |
| 532 | | | min | -128.649 | 1 | -312.975 | 3 | -42.33 | 1 | 0 | 2 | -.105 | 1 | -.003 | 1 |
| 533 | M5 | 1 | max | 297.975 | 1 | 2607.042 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | .022 | 3 |
| 534 | | | min | 6.961 | 12 | -1456.254 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 0 | 15 |
| 535 | | 2 | max | 298.818 | 1 | 2605.948 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | .924 | 2 |
| 536 | | | min | 7.382 | 12 | -1457.713 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | -1.596 | 3 |
| 537 | | 3 | max | 1927.206 | 3 | 1403.632 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | 1.798 | 2 |
| 538 | | | min | -1160.048 | 2 | -1747.273 | 3 | 0 | 1 | 0 | 1 | 0 | 1 | -3.165 | 3 |
| 539 | | 4 | max | 1927.838 | 3 | 1402.173 | 2 | 0 | 1 | 0 | 1 | 0 | 1 | .928 | 2 |





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

| Member | Sec | | Axial[lb] | LC | y Shear[lb] | LC | z Shear[lb] | LC | Torque[k-ft] | LC | y-y Mome... | LC | z-z Mome... | LC |
|--------|-----|-----|-----------|----|-------------|----|-------------|----|--------------|----|-------------|----|-------------|----|
| 597 | 14 | max | 698.427 | 3 | 446.347 | 3 | 35.9 | 1 | 0 | 3 | 0 | 15 | .46 | 3 |
| 598 | | min | -281.393 | 2 | -691.013 | 2 | 1.742 | 15 | 0 | 2 | -.012 | 1 | -.507 | 2 |
| 599 | 15 | max | 699.059 | 3 | 445.253 | 3 | 35.9 | 1 | 0 | 3 | .01 | 1 | .183 | 3 |
| 600 | | min | -280.551 | 2 | -692.472 | 2 | 1.742 | 15 | 0 | 2 | 0 | 15 | -.086 | 1 |
| 601 | 16 | max | 699.691 | 3 | 444.158 | 3 | 35.9 | 1 | 0 | 3 | .033 | 1 | .353 | 2 |
| 602 | | min | -279.709 | 2 | -693.931 | 2 | 1.742 | 15 | 0 | 2 | .002 | 15 | -.093 | 3 |
| 603 | 17 | max | 700.322 | 3 | 443.064 | 3 | 35.9 | 1 | 0 | 3 | .055 | 1 | .784 | 2 |
| 604 | | min | -278.866 | 2 | -695.39 | 2 | 1.742 | 15 | 0 | 2 | .003 | 15 | -.368 | 3 |
| 605 | 18 | max | -6.297 | 15 | 644.783 | 2 | 42.33 | 1 | 0 | 2 | .079 | 1 | .398 | 2 |
| 606 | | min | -129.492 | 1 | -311.881 | 3 | 2.059 | 15 | 0 | 3 | .004 | 15 | -.183 | 3 |
| 607 | 19 | max | -6.043 | 15 | 643.324 | 2 | 42.33 | 1 | 0 | 2 | .105 | 1 | .011 | 3 |
| 608 | | min | -128.649 | 1 | -312.975 | 3 | 2.059 | 15 | 0 | 3 | .005 | 15 | -.003 | 1 |

Envelope Member Section Deflections

| | Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|----|--------|-----|-----|--------|-------|--------|-------|--------|-----------|----------------|----------|---------------|----------|---------------|----|
| 1 | M13 | 1 | max | 0 | 1 | .235 | 2 | .013 | 3 | 1.624e-2 | 2 | NC | 1 | NC | 1 |
| 2 | | | min | 0 | 15 | -.082 | 3 | -.008 | 2 | -5.663e-3 | 3 | NC | 1 | NC | 1 |
| 3 | | 2 | max | 0 | 1 | .198 | 2 | .015 | 3 | 1.686e-2 | 2 | NC | 4 | NC | 1 |
| 4 | | | min | 0 | 15 | .004 | 15 | -.006 | 2 | -4.991e-3 | 3 | 1407.677 | 3 | NC | 1 |
| 5 | | 3 | max | 0 | 1 | .171 | 2 | .018 | 3 | 1.748e-2 | 2 | NC | 4 | NC | 2 |
| 6 | | | min | 0 | 15 | .004 | 15 | -.004 | 10 | -4.32e-3 | 3 | 767.878 | 3 | 7775.076 | 1 |
| 7 | | 4 | max | 0 | 1 | .172 | 3 | .026 | 1 | 1.809e-2 | 2 | NC | 4 | NC | 2 |
| 8 | | | min | 0 | 15 | .003 | 15 | -.005 | 10 | -3.649e-3 | 3 | 589.878 | 3 | 5427.248 | 1 |
| 9 | | 5 | max | 0 | 1 | .196 | 3 | .029 | 1 | 1.871e-2 | 2 | NC | 4 | NC | 2 |
| 10 | | | min | 0 | 15 | .003 | 15 | -.006 | 10 | -2.977e-3 | 3 | 540.423 | 3 | 4848.071 | 1 |
| 11 | | 6 | max | 0 | 1 | .187 | 2 | .029 | 3 | 1.932e-2 | 2 | NC | 4 | NC | 2 |
| 12 | | | min | 0 | 15 | .004 | 15 | -.007 | 10 | -2.306e-3 | 3 | 564.876 | 3 | 5334.57 | 1 |
| 13 | | 7 | max | 0 | 1 | .222 | 2 | .032 | 3 | 1.994e-2 | 2 | NC | 2 | NC | 2 |
| 14 | | | min | 0 | 15 | .004 | 15 | -.01 | 2 | -1.634e-3 | 3 | 667.077 | 3 | 7600.032 | 1 |
| 15 | | 8 | max | 0 | 1 | .264 | 2 | .034 | 3 | 2.055e-2 | 2 | NC | 4 | NC | 1 |
| 16 | | | min | 0 | 15 | .005 | 15 | -.017 | 2 | -9.63e-4 | 3 | 890.97 | 3 | 6997.863 | 3 |
| 17 | | 9 | max | 0 | 1 | .299 | 2 | .035 | 3 | 2.117e-2 | 2 | NC | 4 | NC | 1 |
| 18 | | | min | 0 | 15 | .006 | 15 | -.023 | 2 | -2.916e-4 | 3 | 1303.209 | 3 | 6647.928 | 3 |
| 19 | | 10 | max | 0 | 1 | .315 | 2 | .036 | 3 | 2.179e-2 | 2 | NC | 4 | NC | 1 |
| 20 | | min | 0 | 1 | .006 | 15 | -.025 | 2 | 3.798e-4 | 3 | 1659.353 | 3 | 6547.475 | 3 | |
| 21 | 11 | max | 0 | 15 | .299 | 2 | .035 | 3 | 2.117e-2 | 2 | NC | 4 | NC | 1 | |
| 22 | | min | 0 | 1 | .006 | 15 | -.023 | 2 | -2.916e-4 | 3 | 1303.209 | 3 | 6647.928 | 3 | |
| 23 | 12 | max | 0 | 15 | .264 | 2 | .034 | 3 | 2.055e-2 | 2 | NC | 4 | NC | 1 | |
| 24 | | min | 0 | 1 | .005 | 15 | -.017 | 2 | -9.63e-4 | 3 | 890.97 | 3 | 6997.863 | 3 | |
| 25 | 13 | max | 0 | 15 | .222 | 2 | .032 | 3 | 1.994e-2 | 2 | NC | 2 | NC | 2 | |
| 26 | | min | 0 | 1 | .004 | 15 | -.01 | 2 | -1.634e-3 | 3 | 667.077 | 3 | 7600.032 | 1 | |
| 27 | 14 | max | 0 | 15 | .187 | 2 | .029 | 3 | 1.932e-2 | 2 | NC | 4 | NC | 2 | |
| 28 | | min | 0 | 1 | .004 | 15 | -.007 | 10 | -2.306e-3 | 3 | 564.876 | 3 | 5334.57 | 1 | |
| 29 | 15 | max | 0 | 15 | .196 | 3 | .029 | 1 | 1.871e-2 | 2 | NC | 4 | NC | 2 | |
| 30 | | min | 0 | 1 | .003 | 15 | -.006 | 10 | -2.977e-3 | 3 | 540.423 | 3 | 4848.071 | 1 | |
| 31 | 16 | max | 0 | 15 | .172 | 3 | .026 | 1 | 1.809e-2 | 2 | NC | 4 | NC | 2 | |
| 32 | | min | 0 | 1 | .003 | 15 | -.005 | 10 | -3.649e-3 | 3 | 589.878 | 3 | 5427.248 | 1 | |
| 33 | 17 | max | 0 | 15 | .171 | 2 | .018 | 3 | 1.748e-2 | 2 | NC | 4 | NC | 2 | |
| 34 | | min | 0 | 1 | .004 | 15 | -.004 | 10 | -4.32e-3 | 3 | 767.878 | 3 | 7775.076 | 1 | |
| 35 | 18 | max | 0 | 15 | .198 | 2 | .015 | 3 | 1.686e-2 | 2 | NC | 4 | NC | 1 | |
| 36 | | min | 0 | 1 | .004 | 15 | -.006 | 2 | -4.991e-3 | 3 | 1407.677 | 3 | NC | 1 | |
| 37 | 19 | max | 0 | 15 | .235 | 2 | .013 | 3 | 1.624e-2 | 2 | NC | 1 | NC | 1 | |
| 38 | | min | 0 | 1 | -.082 | 3 | -.008 | 2 | -5.663e-3 | 3 | NC | 1 | NC | 1 | |
| 39 | M14 | 1 | max | 0 | 1 | .511 | 3 | .011 | 3 | 8.711e-3 | 2 | NC | 1 | NC | 1 |
| 40 | | | min | 0 | 15 | -.687 | 2 | -.007 | 2 | -7.483e-3 | 3 | NC | 1 | NC | 1 |



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| | Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|----|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 41 | | 2 | max | 0 | 1 | .664 | 3 | .012 | 3 | 9.702e-3 | 2 | NC | 5 | NC | 1 |
| 42 | | | min | 0 | 15 | -.838 | 2 | -.006 | 2 | -8.444e-3 | 3 | 976.217 | 3 | NC | 1 |
| 43 | | 3 | max | 0 | 1 | .803 | 3 | .015 | 3 | 1.069e-2 | 2 | NC | 5 | NC | 1 |
| 44 | | | min | 0 | 15 | -.976 | 2 | -.004 | 10 | -9.406e-3 | 3 | 513.842 | 3 | NC | 1 |
| 45 | | 4 | max | 0 | 1 | .915 | 3 | .02 | 1 | 1.168e-2 | 2 | NC | 5 | NC | 2 |
| 46 | | | min | 0 | 15 | -1.094 | 2 | -.004 | 10 | -1.037e-2 | 3 | 368.469 | 2 | 6836.74 | 1 |
| 47 | | 5 | max | 0 | 1 | .994 | 3 | .024 | 1 | 1.267e-2 | 2 | NC | 5 | NC | 2 |
| 48 | | | min | 0 | 15 | -1.186 | 2 | -.005 | 10 | -1.133e-2 | 3 | 300.838 | 2 | 5806.964 | 1 |
| 49 | | 6 | max | 0 | 1 | 1.039 | 3 | .025 | 3 | 1.367e-2 | 2 | NC | 5 | NC | 2 |
| 50 | | | min | 0 | 15 | -1.249 | 2 | -.007 | 10 | -1.229e-2 | 3 | 267.026 | 2 | 6181.164 | 1 |
| 51 | | 7 | max | 0 | 1 | 1.053 | 3 | .028 | 3 | 1.466e-2 | 2 | NC | 15 | NC | 2 |
| 52 | | | min | 0 | 15 | -1.285 | 2 | -.009 | 2 | -1.325e-2 | 3 | 250.916 | 2 | 8587.519 | 1 |
| 53 | | 8 | max | 0 | 1 | 1.044 | 3 | .03 | 3 | 1.565e-2 | 2 | NC | 15 | NC | 1 |
| 54 | | | min | 0 | 15 | -1.299 | 2 | -.015 | 2 | -1.421e-2 | 3 | 245.296 | 2 | 8020.151 | 3 |
| 55 | | 9 | max | 0 | 1 | 1.026 | 3 | .031 | 3 | 1.664e-2 | 2 | NC | 15 | NC | 1 |
| 56 | | | min | 0 | 15 | -1.298 | 2 | -.021 | 2 | -1.517e-2 | 3 | 245.528 | 2 | 7552.448 | 3 |
| 57 | | 10 | max | 0 | 1 | 1.015 | 3 | .031 | 3 | 1.763e-2 | 2 | NC | 15 | NC | 1 |
| 58 | | | min | 0 | 1 | -1.295 | 2 | -.023 | 2 | -1.613e-2 | 3 | 246.888 | 2 | 7415.953 | 3 |
| 59 | | 11 | max | 0 | 15 | 1.026 | 3 | .031 | 3 | 1.664e-2 | 2 | NC | 15 | NC | 1 |
| 60 | | | min | 0 | 1 | -1.298 | 2 | -.021 | 2 | -1.517e-2 | 3 | 245.528 | 2 | 7552.448 | 3 |
| 61 | | 12 | max | 0 | 15 | 1.044 | 3 | .03 | 3 | 1.565e-2 | 2 | NC | 15 | NC | 1 |
| 62 | | | min | 0 | 1 | -1.299 | 2 | -.015 | 2 | -1.421e-2 | 3 | 245.296 | 2 | 8020.151 | 3 |
| 63 | | 13 | max | 0 | 15 | 1.053 | 3 | .028 | 3 | 1.466e-2 | 2 | NC | 15 | NC | 2 |
| 64 | | | min | 0 | 1 | -1.285 | 2 | -.009 | 2 | -1.325e-2 | 3 | 250.916 | 2 | 8587.519 | 1 |
| 65 | | 14 | max | 0 | 15 | 1.039 | 3 | .025 | 3 | 1.367e-2 | 2 | NC | 5 | NC | 2 |
| 66 | | | min | 0 | 1 | -1.249 | 2 | -.007 | 10 | -1.229e-2 | 3 | 267.026 | 2 | 6181.164 | 1 |
| 67 | | 15 | max | 0 | 15 | .994 | 3 | .024 | 1 | 1.267e-2 | 2 | NC | 5 | NC | 2 |
| 68 | | | min | 0 | 1 | -1.186 | 2 | -.005 | 10 | -1.133e-2 | 3 | 300.838 | 2 | 5806.964 | 1 |
| 69 | | 16 | max | 0 | 15 | .915 | 3 | .02 | 1 | 1.168e-2 | 2 | NC | 5 | NC | 2 |
| 70 | | | min | 0 | 1 | -1.094 | 2 | -.004 | 10 | -1.037e-2 | 3 | 368.469 | 2 | 6836.74 | 1 |
| 71 | | 17 | max | 0 | 15 | .803 | 3 | .015 | 3 | 1.069e-2 | 2 | NC | 5 | NC | 1 |
| 72 | | | min | 0 | 1 | -.976 | 2 | -.004 | 10 | -9.406e-3 | 3 | 513.842 | 3 | NC | 1 |
| 73 | | 18 | max | 0 | 15 | .664 | 3 | .012 | 3 | 9.702e-3 | 2 | NC | 5 | NC | 1 |
| 74 | | | min | 0 | 1 | -.838 | 2 | -.006 | 2 | -8.444e-3 | 3 | 976.217 | 3 | NC | 1 |
| 75 | | 19 | max | 0 | 15 | .511 | 3 | .011 | 3 | 8.711e-3 | 2 | NC | 1 | NC | 1 |
| 76 | | | min | 0 | 1 | -.687 | 2 | -.007 | 2 | -7.483e-3 | 3 | NC | 1 | NC | 1 |
| 77 | M15 | 1 | max | 0 | 15 | .521 | 3 | .01 | 3 | 6.445e-3 | 3 | NC | 1 | NC | 1 |
| 78 | | | min | 0 | 1 | -.685 | 2 | -.007 | 2 | -9.089e-3 | 2 | NC | 1 | NC | 1 |
| 79 | | 2 | max | 0 | 15 | .645 | 3 | .012 | 3 | 7.259e-3 | 3 | NC | 5 | NC | 1 |
| 80 | | | min | 0 | 1 | -.861 | 2 | -.005 | 2 | -1.013e-2 | 2 | 853.079 | 2 | NC | 1 |
| 81 | | 3 | max | 0 | 15 | .758 | 3 | .014 | 3 | 8.072e-3 | 3 | NC | 5 | NC | 1 |
| 82 | | | min | 0 | 1 | -1.02 | 2 | -.004 | 10 | -1.117e-2 | 2 | 447.851 | 2 | NC | 1 |
| 83 | | 4 | max | 0 | 15 | .856 | 3 | .021 | 1 | 8.885e-3 | 3 | NC | 5 | NC | 2 |
| 84 | | | min | 0 | 1 | -1.151 | 2 | -.004 | 10 | -1.222e-2 | 2 | 322.419 | 2 | 6776.355 | 1 |
| 85 | | 5 | max | 0 | 15 | .933 | 3 | .025 | 1 | 9.698e-3 | 3 | NC | 5 | NC | 2 |
| 86 | | | min | 0 | 1 | -1.245 | 2 | -.005 | 10 | -1.326e-2 | 2 | 267.97 | 2 | 5748.576 | 1 |
| 87 | | 6 | max | 0 | 15 | .987 | 3 | .023 | 3 | 1.051e-2 | 3 | NC | 5 | NC | 2 |
| 88 | | | min | 0 | 1 | -1.302 | 2 | -.006 | 10 | -1.43e-2 | 2 | 243.249 | 2 | 6099.596 | 1 |
| 89 | | 7 | max | 0 | 15 | 1.02 | 3 | .026 | 3 | 1.132e-2 | 3 | NC | 15 | NC | 2 |
| 90 | | | min | 0 | 1 | -1.324 | 2 | -.008 | 2 | -1.535e-2 | 2 | 234.7 | 2 | 8405.302 | 1 |
| 91 | | 8 | max | 0 | 15 | 1.035 | 3 | .028 | 3 | 1.214e-2 | 3 | NC | 15 | NC | 1 |
| 92 | | | min | 0 | 1 | -1.321 | 2 | -.014 | 2 | -1.639e-2 | 2 | 236.049 | 2 | 8651.498 | 3 |
| 93 | | 9 | max | 0 | 15 | 1.037 | 3 | .029 | 3 | 1.295e-2 | 3 | NC | 15 | NC | 1 |
| 94 | | | min | 0 | 1 | -1.304 | 2 | -.019 | 2 | -1.743e-2 | 2 | 242.284 | 2 | 8177.61 | 3 |
| 95 | | 10 | max | 0 | 1 | 1.035 | 3 | .029 | 3 | 1.376e-2 | 3 | NC | 5 | NC | 1 |
| 96 | | | min | 0 | 1 | -1.294 | 2 | -.022 | 2 | -1.847e-2 | 2 | 246.486 | 2 | 8042.794 | 3 |
| 97 | | 11 | max | 0 | 1 | 1.037 | 3 | .029 | 3 | 1.295e-2 | 3 | NC | 15 | NC | 1 |



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| | Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|-----|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 98 | | | min | 0 | 15 | -1.304 | 2 | -.019 | 2 | -1.743e-2 | 2 | 242.284 | 2 | 8177.61 | 3 |
| 99 | | 12 | max | 0 | 1 | 1.035 | 3 | .028 | 3 | 1.214e-2 | 3 | NC | 15 | NC | 1 |
| 100 | | | min | 0 | 15 | -1.321 | 2 | -.014 | 2 | -1.639e-2 | 2 | 236.049 | 2 | 8651.498 | 3 |
| 101 | | 13 | max | 0 | 1 | 1.02 | 3 | .026 | 3 | 1.132e-2 | 3 | NC | 15 | NC | 2 |
| 102 | | | min | 0 | 15 | -1.324 | 2 | -.008 | 2 | -1.535e-2 | 2 | 234.7 | 2 | 8405.302 | 1 |
| 103 | | 14 | max | 0 | 1 | .987 | 3 | .023 | 3 | 1.051e-2 | 3 | NC | 5 | NC | 2 |
| 104 | | | min | 0 | 15 | -1.302 | 2 | -.006 | 10 | -1.43e-2 | 2 | 243.249 | 2 | 6099.596 | 1 |
| 105 | | 15 | max | 0 | 1 | .933 | 3 | .025 | 1 | 9.698e-3 | 3 | NC | 5 | NC | 2 |
| 106 | | | min | 0 | 15 | -1.245 | 2 | -.005 | 10 | -1.326e-2 | 2 | 267.97 | 2 | 5748.576 | 1 |
| 107 | | 16 | max | 0 | 1 | .856 | 3 | .021 | 1 | 8.885e-3 | 3 | NC | 5 | NC | 2 |
| 108 | | | min | 0 | 15 | -1.151 | 2 | -.004 | 10 | -1.222e-2 | 2 | 322.419 | 2 | 6776.355 | 1 |
| 109 | | 17 | max | 0 | 1 | .758 | 3 | .014 | 3 | 8.072e-3 | 3 | NC | 5 | NC | 1 |
| 110 | | | min | 0 | 15 | -1.02 | 2 | -.004 | 10 | -1.117e-2 | 2 | 447.851 | 2 | NC | 1 |
| 111 | | 18 | max | 0 | 1 | .645 | 3 | .012 | 3 | 7.259e-3 | 3 | NC | 5 | NC | 1 |
| 112 | | | min | 0 | 15 | -.861 | 2 | -.005 | 2 | -1.013e-2 | 2 | 853.079 | 2 | NC | 1 |
| 113 | | 19 | max | 0 | 1 | .521 | 3 | .01 | 3 | 6.445e-3 | 3 | NC | 1 | NC | 1 |
| 114 | | | min | 0 | 15 | -.685 | 2 | -.007 | 2 | -9.089e-3 | 2 | NC | 1 | NC | 1 |
| 115 | M16 | 1 | max | 0 | 15 | .209 | 2 | .009 | 3 | 1.266e-2 | 3 | NC | 1 | NC | 1 |
| 116 | | | min | 0 | 1 | -.187 | 3 | -.006 | 2 | -1.369e-2 | 2 | NC | 1 | NC | 1 |
| 117 | | 2 | max | 0 | 15 | .139 | 2 | .011 | 3 | 1.331e-2 | 3 | NC | 4 | NC | 1 |
| 118 | | | min | 0 | 1 | -.165 | 3 | -.003 | 2 | -1.379e-2 | 2 | 2155.102 | 2 | NC | 1 |
| 119 | | 3 | max | 0 | 15 | .085 | 1 | .018 | 1 | 1.397e-2 | 3 | NC | 4 | NC | 2 |
| 120 | | | min | 0 | 1 | -.15 | 3 | -.003 | 10 | -1.388e-2 | 2 | 1199.044 | 2 | 7723.404 | 1 |
| 121 | | 4 | max | 0 | 15 | .065 | 1 | .027 | 1 | 1.462e-2 | 3 | NC | 4 | NC | 2 |
| 122 | | | min | 0 | 1 | -.146 | 3 | -.003 | 10 | -1.398e-2 | 2 | 955.067 | 2 | 5352.834 | 1 |
| 123 | | 5 | max | 0 | 15 | .064 | 1 | .03 | 1 | 1.528e-2 | 3 | NC | 4 | NC | 2 |
| 124 | | | min | 0 | 1 | -.155 | 3 | -.003 | 10 | -1.408e-2 | 2 | 930.902 | 2 | 4738.999 | 1 |
| 125 | | 6 | max | 0 | 15 | .082 | 1 | .028 | 1 | 1.593e-2 | 3 | NC | 3 | NC | 2 |
| 126 | | | min | 0 | 1 | -.177 | 3 | -.005 | 10 | -1.417e-2 | 2 | 1086.291 | 2 | 5138.659 | 1 |
| 127 | | 7 | max | 0 | 15 | .116 | 1 | .023 | 3 | 1.659e-2 | 3 | NC | 4 | NC | 2 |
| 128 | | | min | 0 | 1 | -.207 | 3 | -.007 | 10 | -1.427e-2 | 2 | 1599.874 | 2 | 7088.876 | 1 |
| 129 | | 8 | max | 0 | 15 | .17 | 2 | .024 | 3 | 1.724e-2 | 3 | NC | 1 | NC | 1 |
| 130 | | | min | 0 | 1 | -.241 | 3 | -.012 | 2 | -1.437e-2 | 2 | 2779.581 | 3 | 9821.759 | 3 |
| 131 | | 9 | max | 0 | 15 | .219 | 2 | .025 | 3 | 1.79e-2 | 3 | NC | 2 | NC | 1 |
| 132 | | | min | 0 | 1 | -.27 | 3 | -.017 | 2 | -1.447e-2 | 2 | 1812.436 | 3 | 9463.408 | 3 |
| 133 | | 10 | max | 0 | 1 | .241 | 2 | .025 | 3 | 1.855e-2 | 3 | NC | 4 | NC | 1 |
| 134 | | | min | 0 | 1 | -.283 | 3 | -.02 | 2 | -1.456e-2 | 2 | 1572.618 | 3 | 9379.695 | 3 |
| 135 | | 11 | max | 0 | 1 | .219 | 2 | .025 | 3 | 1.79e-2 | 3 | NC | 2 | NC | 1 |
| 136 | | | min | 0 | 15 | -.27 | 3 | -.017 | 2 | -1.447e-2 | 2 | 1812.436 | 3 | 9463.408 | 3 |
| 137 | | 12 | max | 0 | 1 | .17 | 2 | .024 | 3 | 1.724e-2 | 3 | NC | 1 | NC | 1 |
| 138 | | | min | 0 | 15 | -.241 | 3 | -.012 | 2 | -1.437e-2 | 2 | 2779.581 | 3 | 9821.759 | 3 |
| 139 | | 13 | max | 0 | 1 | .116 | 1 | .023 | 3 | 1.659e-2 | 3 | NC | 4 | NC | 2 |
| 140 | | | min | 0 | 15 | -.207 | 3 | -.007 | 10 | -1.427e-2 | 2 | 1599.874 | 2 | 7088.876 | 1 |
| 141 | | 14 | max | 0 | 1 | .082 | 1 | .028 | 1 | 1.593e-2 | 3 | NC | 3 | NC | 2 |
| 142 | | | min | 0 | 15 | -.177 | 3 | -.005 | 10 | -1.417e-2 | 2 | 1086.291 | 2 | 5138.659 | 1 |
| 143 | | 15 | max | 0 | 1 | .064 | 1 | .03 | 1 | 1.528e-2 | 3 | NC | 4 | NC | 2 |
| 144 | | | min | 0 | 15 | -.155 | 3 | -.003 | 10 | -1.408e-2 | 2 | 930.902 | 2 | 4738.999 | 1 |
| 145 | | 16 | max | 0 | 1 | .065 | 1 | .027 | 1 | 1.462e-2 | 3 | NC | 4 | NC | 2 |
| 146 | | | min | 0 | 15 | -.146 | 3 | -.003 | 10 | -1.398e-2 | 2 | 955.067 | 2 | 5352.834 | 1 |
| 147 | | 17 | max | 0 | 1 | .085 | 1 | .018 | 1 | 1.397e-2 | 3 | NC | 4 | NC | 2 |
| 148 | | | min | 0 | 15 | -.15 | 3 | -.003 | 10 | -1.388e-2 | 2 | 1199.044 | 2 | 7723.404 | 1 |
| 149 | | 18 | max | 0 | 1 | .139 | 2 | .011 | 3 | 1.331e-2 | 3 | NC | 4 | NC | 1 |
| 150 | | | min | 0 | 15 | -.165 | 3 | -.003 | 2 | -1.379e-2 | 2 | 2155.102 | 2 | NC | 1 |
| 151 | | 19 | max | 0 | 1 | .209 | 2 | .009 | 3 | 1.266e-2 | 3 | NC | 1 | NC | 1 |
| 152 | | | min | 0 | 15 | -.187 | 3 | -.006 | 2 | -1.369e-2 | 2 | NC | 1 | NC | 1 |
| 153 | M2 | 1 | max | .008 | 2 | .013 | 2 | .005 | 1 | -5.092e-6 | 15 | NC | 1 | NC | 1 |
| 154 | | | min | -.012 | 3 | -.019 | 3 | 0 | 15 | -1.03e-4 | 1 | 6097.475 | 2 | NC | 1 |



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| | Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|-----|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 155 | | 2 | max | .008 | 2 | .011 | 2 | .004 | 1 | -4.846e-6 | 15 | NC | 1 | NC | 1 |
| 156 | | | min | -.011 | 3 | -.018 | 3 | 0 | 15 | -9.806e-5 | 1 | 7039.053 | 2 | NC | 1 |
| 157 | | 3 | max | .007 | 2 | .009 | 2 | .004 | 1 | -4.6e-6 | 15 | NC | 1 | NC | 1 |
| 158 | | | min | -.01 | 3 | -.018 | 3 | 0 | 15 | -9.308e-5 | 1 | 8305.773 | 2 | NC | 1 |
| 159 | | 4 | max | .007 | 2 | .008 | 2 | .004 | 1 | -4.355e-6 | 15 | NC | 1 | NC | 1 |
| 160 | | | min | -.01 | 3 | -.017 | 3 | 0 | 15 | -8.811e-5 | 1 | NC | 1 | NC | 1 |
| 161 | | 5 | max | .006 | 2 | .006 | 2 | .003 | 1 | -4.109e-6 | 15 | NC | 1 | NC | 1 |
| 162 | | | min | -.009 | 3 | -.016 | 3 | 0 | 15 | -8.313e-5 | 1 | NC | 1 | NC | 1 |
| 163 | | 6 | max | .006 | 2 | .005 | 2 | .003 | 1 | -3.864e-6 | 15 | NC | 1 | NC | 1 |
| 164 | | | min | -.008 | 3 | -.016 | 3 | 0 | 15 | -7.816e-5 | 1 | NC | 1 | NC | 1 |
| 165 | | 7 | max | .006 | 2 | .003 | 2 | .003 | 1 | -3.618e-6 | 15 | NC | 1 | NC | 1 |
| 166 | | | min | -.008 | 3 | -.015 | 3 | 0 | 15 | -7.318e-5 | 1 | NC | 1 | NC | 1 |
| 167 | | 8 | max | .005 | 2 | .002 | 2 | .002 | 1 | -3.372e-6 | 15 | NC | 1 | NC | 1 |
| 168 | | | min | -.007 | 3 | -.014 | 3 | 0 | 15 | -6.82e-5 | 1 | NC | 1 | NC | 1 |
| 169 | | 9 | max | .005 | 2 | 0 | 2 | .002 | 1 | -3.127e-6 | 15 | NC | 1 | NC | 1 |
| 170 | | | min | -.007 | 3 | -.013 | 3 | 0 | 15 | -6.323e-5 | 1 | NC | 1 | NC | 1 |
| 171 | | 10 | max | .004 | 2 | 0 | 2 | .002 | 1 | -2.881e-6 | 15 | NC | 1 | NC | 1 |
| 172 | | | min | -.006 | 3 | -.012 | 3 | 0 | 15 | -5.825e-5 | 1 | NC | 1 | NC | 1 |
| 173 | | 11 | max | .004 | 2 | -.001 | 2 | .001 | 1 | -2.635e-6 | 15 | NC | 1 | NC | 1 |
| 174 | | | min | -.005 | 3 | -.011 | 3 | 0 | 15 | -5.328e-5 | 1 | NC | 1 | NC | 1 |
| 175 | | 12 | max | .003 | 2 | -.002 | 2 | 0 | 1 | -2.39e-6 | 15 | NC | 1 | NC | 1 |
| 176 | | | min | -.005 | 3 | -.01 | 3 | 0 | 15 | -4.83e-5 | 1 | NC | 1 | NC | 1 |
| 177 | | 13 | max | .003 | 2 | -.002 | 15 | 0 | 1 | -2.144e-6 | 15 | NC | 1 | NC | 1 |
| 178 | | | min | -.004 | 3 | -.009 | 3 | 0 | 15 | -4.333e-5 | 1 | NC | 1 | NC | 1 |
| 179 | | 14 | max | .002 | 2 | -.002 | 15 | 0 | 1 | -1.899e-6 | 15 | NC | 1 | NC | 1 |
| 180 | | | min | -.003 | 3 | -.008 | 3 | 0 | 15 | -3.835e-5 | 1 | NC | 1 | NC | 1 |
| 181 | | 15 | max | .002 | 2 | -.001 | 15 | 0 | 1 | -1.653e-6 | 15 | NC | 1 | NC | 1 |
| 182 | | | min | -.003 | 3 | -.006 | 3 | 0 | 15 | -3.337e-5 | 1 | NC | 1 | NC | 1 |
| 183 | | 16 | max | .001 | 2 | -.001 | 15 | 0 | 1 | -1.407e-6 | 15 | NC | 1 | NC | 1 |
| 184 | | | min | -.002 | 3 | -.005 | 3 | 0 | 15 | -2.84e-5 | 1 | NC | 1 | NC | 1 |
| 185 | | 17 | max | 0 | 2 | 0 | 15 | 0 | 1 | -1.162e-6 | 15 | NC | 1 | NC | 1 |
| 186 | | | min | -.001 | 3 | -.003 | 4 | 0 | 15 | -2.342e-5 | 1 | NC | 1 | NC | 1 |
| 187 | | 18 | max | 0 | 2 | 0 | 15 | 0 | 1 | -9.161e-7 | 15 | NC | 1 | NC | 1 |
| 188 | | | min | 0 | 3 | -.002 | 4 | 0 | 15 | -1.845e-5 | 1 | NC | 1 | NC | 1 |
| 189 | | 19 | max | 0 | 1 | 0 | 1 | 0 | 1 | -6.705e-7 | 15 | NC | 1 | NC | 1 |
| 190 | | | min | 0 | 1 | 0 | 1 | 0 | 1 | -1.347e-5 | 1 | NC | 1 | NC | 1 |
| 191 | M3 | 1 | max | 0 | 1 | 0 | 1 | 0 | 1 | 2.77e-6 | 1 | NC | 1 | NC | 1 |
| 192 | | | min | 0 | 1 | 0 | 1 | 0 | 1 | 1.382e-7 | 15 | NC | 1 | NC | 1 |
| 193 | | 2 | max | 0 | 3 | 0 | 15 | 0 | 15 | 1.511e-5 | 1 | NC | 1 | NC | 1 |
| 194 | | | min | 0 | 2 | -.003 | 4 | 0 | 1 | 7.339e-7 | 15 | NC | 1 | NC | 1 |
| 195 | | 3 | max | .001 | 3 | -.001 | 15 | 0 | 15 | 2.745e-5 | 1 | NC | 1 | NC | 1 |
| 196 | | | min | 0 | 2 | -.006 | 4 | 0 | 1 | 1.33e-6 | 15 | NC | 1 | NC | 1 |
| 197 | | 4 | max | .002 | 3 | -.002 | 15 | 0 | 15 | 3.979e-5 | 1 | NC | 1 | NC | 1 |
| 198 | | | min | -.001 | 2 | -.009 | 4 | 0 | 1 | 1.925e-6 | 15 | NC | 1 | NC | 1 |
| 199 | | 5 | max | .002 | 3 | -.003 | 15 | 0 | 10 | 5.213e-5 | 1 | NC | 1 | NC | 1 |
| 200 | | | min | -.002 | 2 | -.012 | 4 | 0 | 1 | 2.521e-6 | 15 | 8395.854 | 4 | NC | 1 |
| 201 | | 6 | max | .003 | 3 | -.004 | 15 | 0 | 10 | 6.447e-5 | 1 | NC | 2 | NC | 1 |
| 202 | | | min | -.002 | 2 | -.015 | 4 | 0 | 1 | 3.116e-6 | 15 | 6812.404 | 4 | NC | 1 |
| 203 | | 7 | max | .003 | 3 | -.004 | 15 | 0 | 10 | 7.682e-5 | 1 | NC | 5 | NC | 1 |
| 204 | | | min | -.003 | 2 | -.018 | 4 | 0 | 1 | 3.712e-6 | 15 | 5858.341 | 4 | NC | 1 |
| 205 | | 8 | max | .004 | 3 | -.005 | 15 | 0 | 10 | 8.916e-5 | 1 | NC | 5 | NC | 1 |
| 206 | | | min | -.003 | 2 | -.02 | 4 | 0 | 1 | 4.308e-6 | 15 | 5270.075 | 4 | NC | 1 |
| 207 | | 9 | max | .004 | 3 | -.005 | 15 | 0 | 1 | 1.015e-4 | 1 | NC | 5 | NC | 1 |
| 208 | | | min | -.004 | 2 | -.021 | 4 | 0 | 3 | 4.903e-6 | 15 | 4923.51 | 4 | NC | 1 |
| 209 | | 10 | max | .005 | 3 | -.005 | 15 | 0 | 1 | 1.138e-4 | 1 | NC | 5 | NC | 1 |
| 210 | | | min | -.004 | 2 | -.022 | 4 | 0 | 3 | 5.499e-6 | 15 | 4758.541 | 4 | NC | 1 |
| 211 | | 11 | max | .006 | 3 | -.005 | 15 | 0 | 1 | 1.262e-4 | 1 | NC | 5 | NC | 1 |



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

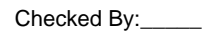
Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 212 | | min | -.005 | 2 | -.022 | 4 | 0 | 15 | 6.095e-6 | 15 | 4750.952 | 4 | NC | 1 |
| 213 | | max | .006 | 3 | -.005 | 15 | 0 | 1 | 1.385e-4 | 1 | NC | 5 | NC | 1 |
| 214 | | min | -.005 | 2 | -.021 | 4 | 0 | 15 | 6.69e-6 | 15 | 4903.13 | 4 | NC | 1 |
| 215 | | max | .007 | 3 | -.005 | 15 | .001 | 1 | 1.509e-4 | 1 | NC | 5 | NC | 1 |
| 216 | | min | -.006 | 2 | -.02 | 4 | 0 | 15 | 7.286e-6 | 15 | 5246.185 | 4 | NC | 1 |
| 217 | | max | .007 | 3 | -.004 | 15 | .001 | 1 | 1.632e-4 | 1 | NC | 5 | NC | 1 |
| 218 | | min | -.006 | 2 | -.018 | 4 | 0 | 15 | 7.882e-6 | 15 | 5855.955 | 4 | NC | 1 |
| 219 | | max | .008 | 3 | -.004 | 15 | .002 | 1 | 1.755e-4 | 1 | NC | 3 | NC | 1 |
| 220 | | min | -.007 | 2 | -.015 | 4 | 0 | 15 | 8.477e-6 | 15 | 6900.076 | 4 | NC | 1 |
| 221 | | max | .008 | 3 | -.003 | 15 | .002 | 1 | 1.879e-4 | 1 | NC | 1 | NC | 1 |
| 222 | | min | -.007 | 2 | -.012 | 4 | 0 | 15 | 9.073e-6 | 15 | 8784.208 | 4 | NC | 1 |
| 223 | | max | .009 | 3 | -.002 | 15 | .003 | 1 | 2.002e-4 | 1 | NC | 1 | NC | 1 |
| 224 | | min | -.008 | 2 | -.008 | 4 | 0 | 15 | 9.669e-6 | 15 | NC | 1 | NC | 1 |
| 225 | | max | .009 | 3 | -.001 | 15 | .004 | 1 | 2.126e-4 | 1 | NC | 1 | NC | 1 |
| 226 | | min | -.008 | 2 | -.005 | 3 | 0 | 15 | 1.026e-5 | 15 | NC | 1 | NC | 1 |
| 227 | | max | .01 | 3 | 0 | 10 | .005 | 1 | 2.249e-4 | 1 | NC | 1 | NC | 1 |
| 228 | | min | -.009 | 2 | -.002 | 3 | 0 | 15 | 1.086e-5 | 15 | NC | 1 | NC | 1 |
| 229 | M4 | max | .002 | 1 | .009 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 230 | | min | 0 | 3 | -.01 | 3 | -.005 | 1 | 4.314e-6 | 15 | NC | 1 | 5471.776 | 1 |
| 231 | | max | .002 | 1 | .008 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 232 | | min | 0 | 3 | -.01 | 3 | -.004 | 1 | 4.314e-6 | 15 | NC | 1 | 5935.943 | 1 |
| 233 | | max | .002 | 1 | .008 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 234 | | min | 0 | 3 | -.009 | 3 | -.004 | 1 | 4.314e-6 | 15 | NC | 1 | 6489.271 | 1 |
| 235 | | max | .002 | 1 | .007 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 236 | | min | 0 | 3 | -.009 | 3 | -.003 | 1 | 4.314e-6 | 15 | NC | 1 | 7154.765 | 1 |
| 237 | | max | .002 | 1 | .007 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 238 | | min | 0 | 3 | -.008 | 3 | -.003 | 1 | 4.314e-6 | 15 | NC | 1 | 7963.697 | 1 |
| 239 | | max | .001 | 1 | .006 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 2 |
| 240 | | min | 0 | 3 | -.007 | 3 | -.003 | 1 | 4.314e-6 | 15 | NC | 1 | 8959.442 | 1 |
| 241 | | max | .001 | 1 | .006 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 242 | | min | 0 | 3 | -.007 | 3 | -.002 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 243 | | max | .001 | 1 | .005 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 244 | | min | 0 | 3 | -.006 | 3 | -.002 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 245 | | max | .001 | 1 | .005 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 246 | | min | 0 | 3 | -.006 | 3 | -.002 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 247 | | max | .001 | 1 | .004 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 248 | | min | 0 | 3 | -.005 | 3 | -.001 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 249 | | max | 0 | 1 | .004 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 250 | | min | 0 | 3 | -.005 | 3 | -.001 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 251 | | max | 0 | 1 | .003 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 252 | | min | 0 | 3 | -.004 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 253 | | max | 0 | 1 | .003 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 254 | | min | 0 | 3 | -.003 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 255 | | max | 0 | 1 | .002 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 256 | | min | 0 | 3 | -.003 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 257 | | max | 0 | 1 | .002 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 258 | | min | 0 | 3 | -.002 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 259 | | max | 0 | 1 | .001 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 260 | | min | 0 | 3 | -.002 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 261 | | max | 0 | 1 | 0 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 262 | | min | 0 | 3 | -.001 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 263 | | max | 0 | 1 | 0 | 2 | 0 | 15 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 264 | | min | 0 | 3 | 0 | 3 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 265 | | max | 0 | 1 | 0 | 1 | 0 | 1 | 8.861e-5 | 1 | NC | 1 | NC | 1 |
| 266 | | min | 0 | 1 | 0 | 1 | 0 | 1 | 4.314e-6 | 15 | NC | 1 | NC | 1 |
| 267 | M6 | max | .024 | 2 | .038 | 2 | 0 | 1 | 0 | 1 | NC | 3 | NC | 1 |
| 268 | | min | -.035 | 3 | -.055 | 3 | 0 | 1 | 0 | 1 | 2018.888 | 2 | NC | 1 |







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 383 | 2 | max | .008 | 2 | .011 | 2 | 0 | 15 | 9.806e-5 | 1 | NC | 1 | NC | 1 |
| 384 | | min | -.011 | 3 | -.018 | 3 | -.004 | 1 | 4.846e-6 | 15 | 7039.053 | 2 | NC | 1 |
| 385 | 3 | max | .007 | 2 | .009 | 2 | 0 | 15 | 9.308e-5 | 1 | NC | 1 | NC | 1 |
| 386 | | min | -.01 | 3 | -.018 | 3 | -.004 | 1 | 4.6e-6 | 15 | 8305.773 | 2 | NC | 1 |
| 387 | 4 | max | .007 | 2 | .008 | 2 | 0 | 15 | 8.811e-5 | 1 | NC | 1 | NC | 1 |
| 388 | | min | -.01 | 3 | -.017 | 3 | -.004 | 1 | 4.355e-6 | 15 | NC | 1 | NC | 1 |
| 389 | 5 | max | .006 | 2 | .006 | 2 | 0 | 15 | 8.313e-5 | 1 | NC | 1 | NC | 1 |
| 390 | | min | -.009 | 3 | -.016 | 3 | -.003 | 1 | 4.109e-6 | 15 | NC | 1 | NC | 1 |
| 391 | 6 | max | .006 | 2 | .005 | 2 | 0 | 15 | 7.816e-5 | 1 | NC | 1 | NC | 1 |
| 392 | | min | -.008 | 3 | -.016 | 3 | -.003 | 1 | 3.864e-6 | 15 | NC | 1 | NC | 1 |
| 393 | 7 | max | .006 | 2 | .003 | 2 | 0 | 15 | 7.318e-5 | 1 | NC | 1 | NC | 1 |
| 394 | | min | -.008 | 3 | -.015 | 3 | -.003 | 1 | 3.618e-6 | 15 | NC | 1 | NC | 1 |
| 395 | 8 | max | .005 | 2 | .002 | 2 | 0 | 15 | 6.82e-5 | 1 | NC | 1 | NC | 1 |
| 396 | | min | -.007 | 3 | -.014 | 3 | -.002 | 1 | 3.372e-6 | 15 | NC | 1 | NC | 1 |
| 397 | 9 | max | .005 | 2 | 0 | 2 | 0 | 15 | 6.323e-5 | 1 | NC | 1 | NC | 1 |
| 398 | | min | -.007 | 3 | -.013 | 3 | -.002 | 1 | 3.127e-6 | 15 | NC | 1 | NC | 1 |
| 399 | 10 | max | .004 | 2 | 0 | 2 | 0 | 15 | 5.825e-5 | 1 | NC | 1 | NC | 1 |
| 400 | | min | -.006 | 3 | -.012 | 3 | -.002 | 1 | 2.881e-6 | 15 | NC | 1 | NC | 1 |
| 401 | 11 | max | .004 | 2 | -.001 | 2 | 0 | 15 | 5.328e-5 | 1 | NC | 1 | NC | 1 |
| 402 | | min | -.005 | 3 | -.011 | 3 | -.001 | 1 | 2.635e-6 | 15 | NC | 1 | NC | 1 |
| 403 | 12 | max | .003 | 2 | -.002 | 2 | 0 | 15 | 4.83e-5 | 1 | NC | 1 | NC | 1 |
| 404 | | min | -.005 | 3 | -.01 | 3 | 0 | 1 | 2.39e-6 | 15 | NC | 1 | NC | 1 |
| 405 | 13 | max | .003 | 2 | -.002 | 15 | 0 | 15 | 4.333e-5 | 1 | NC | 1 | NC | 1 |
| 406 | | min | -.004 | 3 | -.009 | 3 | 0 | 1 | 2.144e-6 | 15 | NC | 1 | NC | 1 |
| 407 | 14 | max | .002 | 2 | -.002 | 15 | 0 | 15 | 3.835e-5 | 1 | NC | 1 | NC | 1 |
| 408 | | min | -.003 | 3 | -.008 | 3 | 0 | 1 | 1.899e-6 | 15 | NC | 1 | NC | 1 |
| 409 | 15 | max | .002 | 2 | -.001 | 15 | 0 | 15 | 3.337e-5 | 1 | NC | 1 | NC | 1 |
| 410 | | min | -.003 | 3 | -.006 | 3 | 0 | 1 | 1.653e-6 | 15 | NC | 1 | NC | 1 |
| 411 | 16 | max | .001 | 2 | -.001 | 15 | 0 | 15 | 2.84e-5 | 1 | NC | 1 | NC | 1 |
| 412 | | min | -.002 | 3 | -.005 | 3 | 0 | 1 | 1.407e-6 | 15 | NC | 1 | NC | 1 |
| 413 | 17 | max | 0 | 2 | 0 | 15 | 0 | 15 | 2.342e-5 | 1 | NC | 1 | NC | 1 |
| 414 | | min | -.001 | 3 | -.003 | 4 | 0 | 1 | 1.162e-6 | 15 | NC | 1 | NC | 1 |
| 415 | 18 | max | 0 | 2 | 0 | 15 | 0 | 15 | 1.845e-5 | 1 | NC | 1 | NC | 1 |
| 416 | | min | 0 | 3 | -.002 | 4 | 0 | 1 | 9.161e-7 | 15 | NC | 1 | NC | 1 |
| 417 | 19 | max | 0 | 1 | 0 | 1 | 0 | 1 | 1.347e-5 | 1 | NC | 1 | NC | 1 |
| 418 | | min | 0 | 1 | 0 | 1 | 0 | 1 | 6.705e-7 | 15 | NC | 1 | NC | 1 |
| 419 | M11 | 1 | max | 0 | 1 | 0 | 1 | 1 | -1.382e-7 | 15 | NC | 1 | NC | 1 |
| 420 | | min | 0 | 1 | 0 | 1 | 0 | 1 | -2.77e-6 | 1 | NC | 1 | NC | 1 |
| 421 | 2 | max | 0 | 3 | 0 | 15 | 0 | 1 | -7.339e-7 | 15 | NC | 1 | NC | 1 |
| 422 | | min | 0 | 2 | -.003 | 4 | 0 | 15 | -1.511e-5 | 1 | NC | 1 | NC | 1 |
| 423 | 3 | max | .001 | 3 | -.001 | 15 | 0 | 1 | -1.33e-6 | 15 | NC | 1 | NC | 1 |
| 424 | | min | 0 | 2 | -.006 | 4 | 0 | 15 | -2.745e-5 | 1 | NC | 1 | NC | 1 |
| 425 | 4 | max | .002 | 3 | -.002 | 15 | 0 | 1 | -1.925e-6 | 15 | NC | 1 | NC | 1 |
| 426 | | min | -.001 | 2 | -.009 | 4 | 0 | 15 | -3.979e-5 | 1 | NC | 1 | NC | 1 |
| 427 | 5 | max | .002 | 3 | -.003 | 15 | 0 | 1 | -2.521e-6 | 15 | NC | 1 | NC | 1 |
| 428 | | min | -.002 | 2 | -.012 | 4 | 0 | 10 | -5.213e-5 | 1 | 8395.854 | 4 | NC | 1 |
| 429 | 6 | max | .003 | 3 | -.004 | 15 | 0 | 1 | -3.116e-6 | 15 | NC | 2 | NC | 1 |
| 430 | | min | -.002 | 2 | -.015 | 4 | 0 | 10 | -6.447e-5 | 1 | 6812.404 | 4 | NC | 1 |
| 431 | 7 | max | .003 | 3 | -.004 | 15 | 0 | 1 | -3.712e-6 | 15 | NC | 5 | NC | 1 |
| 432 | | min | -.003 | 2 | -.018 | 4 | 0 | 10 | -7.682e-5 | 1 | 5858.341 | 4 | NC | 1 |
| 433 | 8 | max | .004 | 3 | -.005 | 15 | 0 | 1 | -4.308e-6 | 15 | NC | 5 | NC | 1 |
| 434 | | min | -.003 | 2 | -.02 | 4 | 0 | 10 | -8.916e-5 | 1 | 5270.075 | 4 | NC | 1 |
| 435 | 9 | max | .004 | 3 | -.005 | 15 | 0 | 3 | -4.903e-6 | 15 | NC | 5 | NC | 1 |
| 436 | | min | -.004 | 2 | -.021 | 4 | 0 | 1 | -1.015e-4 | 1 | 4923.51 | 4 | NC | 1 |
| 437 | 10 | max | .005 | 3 | -.005 | 15 | 0 | 3 | -5.499e-6 | 15 | NC | 5 | NC | 1 |
| 438 | | min | -.004 | 2 | -.022 | 4 | 0 | 1 | -1.138e-4 | 1 | 4758.541 | 4 | NC | 1 |
| 439 | 11 | max | .006 | 3 | -.005 | 15 | 0 | 15 | -6.095e-6 | 15 | NC | 5 | NC | 1 |



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 440 | | min | -.005 | 2 | -.022 | 4 | 0 | 1 | -1.262e-4 | 1 | 4750.952 | 4 | NC | 1 |
| 441 | | max | .006 | 3 | -.005 | 15 | 0 | 15 | -6.69e-6 | 15 | NC | 5 | NC | 1 |
| 442 | | min | -.005 | 2 | -.021 | 4 | 0 | 1 | -1.385e-4 | 1 | 4903.13 | 4 | NC | 1 |
| 443 | | max | .007 | 3 | -.005 | 15 | 0 | 15 | -7.286e-6 | 15 | NC | 5 | NC | 1 |
| 444 | | min | -.006 | 2 | -.02 | 4 | -.001 | 1 | -1.509e-4 | 1 | 5246.185 | 4 | NC | 1 |
| 445 | | max | .007 | 3 | -.004 | 15 | 0 | 15 | -7.882e-6 | 15 | NC | 5 | NC | 1 |
| 446 | | min | -.006 | 2 | -.018 | 4 | -.001 | 1 | -1.632e-4 | 1 | 5855.955 | 4 | NC | 1 |
| 447 | | max | .008 | 3 | -.004 | 15 | 0 | 15 | -8.477e-6 | 15 | NC | 3 | NC | 1 |
| 448 | | min | -.007 | 2 | -.015 | 4 | -.002 | 1 | -1.755e-4 | 1 | 6900.076 | 4 | NC | 1 |
| 449 | | max | .008 | 3 | -.003 | 15 | 0 | 15 | -9.073e-6 | 15 | NC | 1 | NC | 1 |
| 450 | | min | -.007 | 2 | -.012 | 4 | -.002 | 1 | -1.879e-4 | 1 | 8784.208 | 4 | NC | 1 |
| 451 | | max | .009 | 3 | -.002 | 15 | 0 | 15 | -9.669e-6 | 15 | NC | 1 | NC | 1 |
| 452 | | min | -.008 | 2 | -.008 | 4 | -.003 | 1 | -2.002e-4 | 1 | NC | 1 | NC | 1 |
| 453 | | max | .009 | 3 | -.001 | 15 | 0 | 15 | -1.026e-5 | 15 | NC | 1 | NC | 1 |
| 454 | | min | -.008 | 2 | -.005 | 3 | -.004 | 1 | -2.126e-4 | 1 | NC | 1 | NC | 1 |
| 455 | | max | .01 | 3 | 0 | 10 | 0 | 15 | -1.086e-5 | 15 | NC | 1 | NC | 1 |
| 456 | | min | -.009 | 2 | -.002 | 3 | -.005 | 1 | -2.249e-4 | 1 | NC | 1 | NC | 1 |
| 457 | M12 | max | .002 | 1 | .009 | 2 | .005 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 458 | | min | 0 | 3 | -.01 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 5471.776 | 1 |
| 459 | | max | .002 | 1 | .008 | 2 | .004 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 460 | | min | 0 | 3 | -.01 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 5935.943 | 1 |
| 461 | | max | .002 | 1 | .008 | 2 | .004 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 462 | | min | 0 | 3 | -.009 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 6489.271 | 1 |
| 463 | | max | .002 | 1 | .007 | 2 | .003 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 464 | | min | 0 | 3 | -.009 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 7154.765 | 1 |
| 465 | | max | .002 | 1 | .007 | 2 | .003 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 466 | | min | 0 | 3 | -.008 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 7963.697 | 1 |
| 467 | | max | .001 | 1 | .006 | 2 | .003 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 2 |
| 468 | | min | 0 | 3 | -.007 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | 8959.442 | 1 |
| 469 | | max | .001 | 1 | .006 | 2 | .002 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 470 | | min | 0 | 3 | -.007 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 471 | | max | .001 | 1 | .005 | 2 | .002 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 472 | | min | 0 | 3 | -.006 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 473 | | max | .001 | 1 | .005 | 2 | .002 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 474 | | min | 0 | 3 | -.006 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 475 | | max | .001 | 1 | .004 | 2 | .001 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 476 | | min | 0 | 3 | -.005 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 477 | | max | 0 | 1 | .004 | 2 | .001 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 478 | | min | 0 | 3 | -.005 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 479 | | max | 0 | 1 | .003 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 480 | | min | 0 | 3 | -.004 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 481 | | max | 0 | 1 | .003 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 482 | | min | 0 | 3 | -.003 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 483 | | max | 0 | 1 | .002 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 484 | | min | 0 | 3 | -.003 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 485 | | max | 0 | 1 | .002 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 486 | | min | 0 | 3 | -.002 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 487 | | max | 0 | 1 | .001 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 488 | | min | 0 | 3 | -.002 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 489 | | max | 0 | 1 | 0 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 490 | | min | 0 | 3 | -.001 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 491 | | max | 0 | 1 | 0 | 2 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 492 | | min | 0 | 3 | 0 | 3 | 0 | 15 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 493 | | max | 0 | 1 | 0 | 1 | 0 | 1 | -4.314e-6 | 15 | NC | 1 | NC | 1 |
| 494 | | min | 0 | 1 | 0 | 1 | 0 | 1 | -8.861e-5 | 1 | NC | 1 | NC | 1 |
| 495 | M1 | max | .013 | 3 | .235 | 2 | 0 | 1 | 4.632e-3 | 2 | NC | 1 | NC | 1 |
| 496 | | min | -.008 | 2 | -.082 | 3 | 0 | 15 | -1.324e-2 | 3 | NC | 1 | NC | 1 |





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

| | Member | Sec | | x [in] | LC | y [in] | LC | z [in] | LC | x Rotate [r... | LC | (n) L/y Ratio | LC | (n) L/z Ratio | LC |
|-----|--------|-----|-----|--------|----|--------|----|--------|----|----------------|----|---------------|----|---------------|----|
| 554 | | | min | -.022 | 2 | -1.36 | 2 | 0 | 1 | 0 | 1 | 82.036 | 2 | NC | 1 |
| 555 | | 12 | max | .029 | 3 | .989 | 3 | 0 | 1 | 0 | 1 | 4680.683 | 15 | NC | 1 |
| 556 | | | min | -.022 | 2 | -1.231 | 2 | 0 | 1 | 0 | 1 | 89.327 | 2 | NC | 1 |
| 557 | | 13 | max | .028 | 3 | .834 | 3 | 0 | 1 | 0 | 1 | 5348.669 | 15 | NC | 1 |
| 558 | | | min | -.021 | 2 | -1.024 | 2 | 0 | 1 | 0 | 1 | 103.948 | 2 | NC | 1 |
| 559 | | 14 | max | .027 | 3 | .64 | 3 | 0 | 1 | 0 | 1 | 6506.18 | 15 | NC | 1 |
| 560 | | | min | -.021 | 2 | -.769 | 2 | 0 | 1 | 0 | 1 | 130.045 | 2 | NC | 1 |
| 561 | | 15 | max | .026 | 3 | .427 | 3 | 0 | 1 | 0 | 1 | 8530.51 | 15 | NC | 1 |
| 562 | | | min | -.02 | 2 | -.497 | 2 | 0 | 1 | 0 | 1 | 177.663 | 2 | NC | 1 |
| 563 | | 16 | max | .026 | 3 | .213 | 3 | 0 | 1 | 0 | 1 | NC | 15 | NC | 1 |
| 564 | | | min | -.02 | 2 | -.238 | 2 | 0 | 1 | 0 | 1 | 273.031 | 2 | NC | 1 |
| 565 | | 17 | max | .025 | 3 | .018 | 3 | 0 | 1 | 0 | 1 | NC | 5 | NC | 1 |
| 566 | | | min | -.02 | 2 | -.021 | 2 | 0 | 1 | 0 | 1 | 495.658 | 2 | NC | 1 |
| 567 | | 18 | max | .025 | 3 | .129 | 2 | 0 | 1 | 0 | 1 | NC | 5 | NC | 1 |
| 568 | | | min | -.02 | 2 | -.142 | 3 | 0 | 1 | 0 | 1 | 1078.35 | 3 | NC | 1 |
| 569 | | 19 | max | .025 | 3 | .241 | 2 | 0 | 1 | 0 | 1 | NC | 1 | NC | 1 |
| 570 | | | min | -.02 | 2 | -.283 | 3 | 0 | 1 | 0 | 1 | NC | 1 | NC | 1 |
| 571 | M9 | 1 | max | .013 | 3 | .235 | 2 | 0 | 15 | 1.324e-2 | 3 | NC | 1 | NC | 1 |
| 572 | | | min | -.008 | 2 | -.082 | 3 | 0 | 1 | -4.632e-3 | 2 | NC | 1 | NC | 1 |
| 573 | | 2 | max | .013 | 3 | .113 | 2 | .003 | 1 | 6.575e-3 | 3 | NC | 5 | NC | 1 |
| 574 | | | min | -.008 | 2 | -.038 | 3 | 0 | 15 | -2.275e-3 | 2 | 1115.775 | 2 | NC | 1 |
| 575 | | 3 | max | .013 | 3 | .02 | 3 | .005 | 1 | 9.283e-5 | 3 | NC | 5 | NC | 1 |
| 576 | | | min | -.008 | 2 | -.015 | 2 | 0 | 15 | -2.614e-5 | 10 | 542.263 | 2 | NC | 1 |
| 577 | | 4 | max | .012 | 3 | .103 | 3 | .004 | 1 | 3.557e-3 | 3 | NC | 5 | NC | 1 |
| 578 | | | min | -.008 | 2 | -.156 | 2 | 0 | 15 | -3.414e-3 | 2 | 346.948 | 2 | NC | 1 |
| 579 | | 5 | max | .012 | 3 | .202 | 3 | .003 | 1 | 7.022e-3 | 3 | NC | 15 | NC | 1 |
| 580 | | | min | -.008 | 2 | -.3 | 2 | 0 | 15 | -6.807e-3 | 2 | 253.211 | 2 | NC | 1 |
| 581 | | 6 | max | .012 | 3 | .306 | 3 | .001 | 1 | 1.049e-2 | 3 | 9636.155 | 15 | NC | 1 |
| 582 | | | min | -.008 | 2 | -.439 | 2 | 0 | 15 | -1.02e-2 | 2 | 201.159 | 2 | NC | 1 |
| 583 | | 7 | max | .012 | 3 | .405 | 3 | 0 | 3 | 1.395e-2 | 3 | 8170.954 | 15 | NC | 1 |
| 584 | | | min | -.008 | 2 | -.561 | 2 | 0 | 1 | -1.359e-2 | 2 | 170.234 | 2 | NC | 1 |
| 585 | | 8 | max | .011 | 3 | .486 | 3 | 0 | 15 | 1.742e-2 | 3 | 7300.858 | 15 | NC | 1 |
| 586 | | | min | -.007 | 2 | -.658 | 2 | 0 | 1 | -1.699e-2 | 2 | 151.862 | 2 | NC | 1 |
| 587 | | 9 | max | .011 | 3 | .538 | 3 | 0 | 1 | 1.809e-2 | 3 | 6843.942 | 15 | NC | 1 |
| 588 | | | min | -.007 | 2 | -.719 | 2 | 0 | 15 | -1.894e-2 | 2 | 142.261 | 2 | NC | 1 |
| 589 | | 10 | max | .011 | 3 | .558 | 3 | 0 | 10 | 1.689e-2 | 3 | 6703.722 | 15 | NC | 1 |
| 590 | | | min | -.007 | 2 | -.739 | 2 | 0 | 1 | -1.992e-2 | 2 | 139.461 | 2 | NC | 1 |
| 591 | | 11 | max | .011 | 3 | .545 | 3 | 0 | 15 | 1.57e-2 | 3 | 6843.255 | 15 | NC | 1 |
| 592 | | | min | -.007 | 2 | -.718 | 2 | 0 | 1 | -2.09e-2 | 2 | 142.811 | 2 | NC | 1 |
| 593 | | 12 | max | .01 | 3 | .5 | 3 | 0 | 1 | 1.387e-2 | 3 | 7299.359 | 15 | NC | 1 |
| 594 | | | min | -.007 | 2 | -.655 | 2 | 0 | 15 | -1.991e-2 | 2 | 153.44 | 2 | NC | 1 |
| 595 | | 13 | max | .01 | 3 | .427 | 3 | 0 | 1 | 1.11e-2 | 3 | 8168.274 | 15 | NC | 1 |
| 596 | | | min | -.007 | 2 | -.553 | 2 | 0 | 10 | -1.596e-2 | 2 | 173.89 | 2 | NC | 1 |
| 597 | | 14 | max | .01 | 3 | .333 | 3 | 0 | 15 | 8.323e-3 | 3 | 9631.574 | 15 | NC | 1 |
| 598 | | | min | -.007 | 2 | -.426 | 2 | -.001 | 1 | -1.201e-2 | 2 | 208.698 | 2 | NC | 1 |
| 599 | | 15 | max | .009 | 3 | .227 | 3 | 0 | 15 | 5.549e-3 | 3 | NC | 15 | NC | 1 |
| 600 | | | min | -.007 | 2 | -.285 | 2 | -.003 | 1 | -8.061e-3 | 2 | 268.239 | 2 | NC | 1 |
| 601 | | 16 | max | .009 | 3 | .116 | 3 | 0 | 15 | 2.775e-3 | 3 | NC | 5 | NC | 1 |
| 602 | | | min | -.007 | 2 | -.142 | 2 | -.004 | 1 | -4.111e-3 | 2 | 377.568 | 2 | NC | 1 |
| 603 | | 17 | max | .009 | 3 | .007 | 3 | 0 | 15 | 1.261e-6 | 3 | NC | 5 | NC | 1 |
| 604 | | | min | -.006 | 2 | -.008 | 2 | -.005 | 1 | -3.161e-4 | 1 | 609.076 | 2 | NC | 1 |
| 605 | | 18 | max | .009 | 3 | .106 | 2 | 0 | 15 | 1.377e-3 | 3 | NC | 5 | NC | 1 |
| 606 | | | min | -.006 | 2 | -.093 | 3 | -.003 | 1 | -4.365e-3 | 2 | 1282.443 | 2 | NC | 1 |
| 607 | | 19 | max | .009 | 3 | .209 | 2 | 0 | 1 | 2.828e-3 | 3 | NC | 1 | NC | 1 |
| 608 | | | min | -.006 | 2 | -.187 | 3 | 0 | 15 | -8.715e-3 | 2 | NC | 1 | NC | 1 |



Anchor Designer™
Software
Version 2.4.6025.0

| | | | |
|-----------|---|-------|----------|
| Company: | Schletter, Inc. | Date: | 8/1/2016 |
| Engineer: | HCV | Page: | 1/5 |
| Project: | Standard PVMax - Worst Case, 14-40 Inch Width | | |
| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

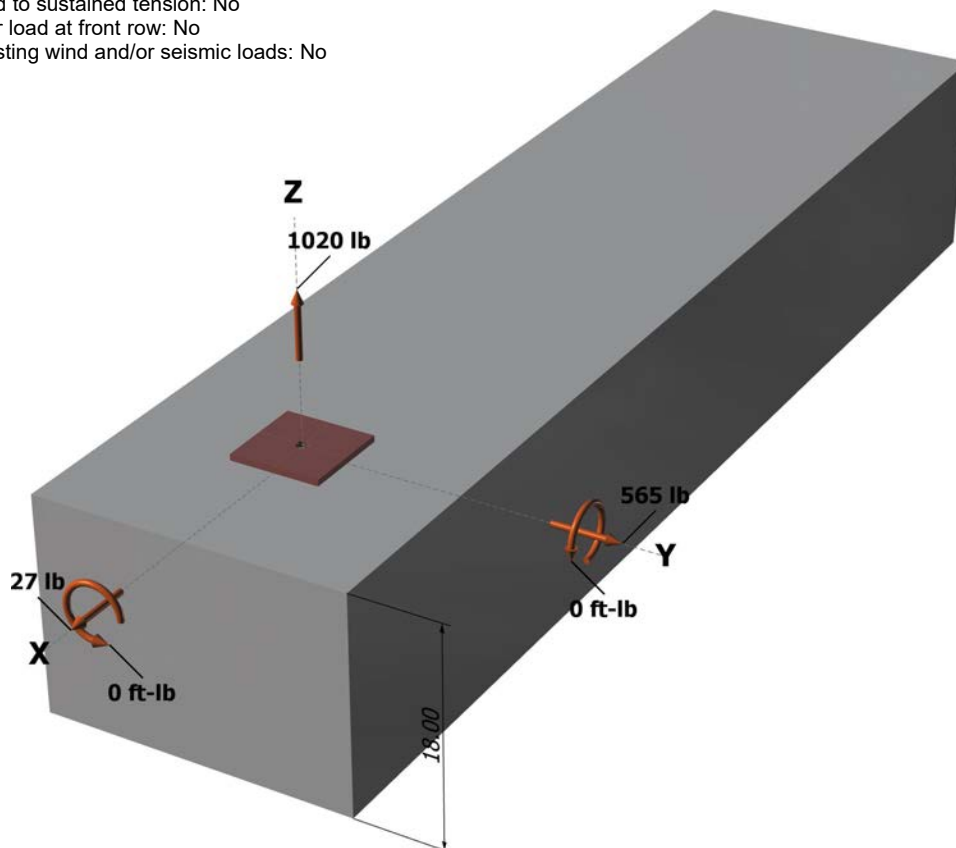
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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|-----------|---|-------|----------|
| Company: | Schletter, Inc. | Date: | 8/1/2016 |
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| Project: | Standard PVMax - Worst Case, 14-40 Inch Width | | |
| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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| | | | |
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| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

3. Resulting Anchor Forces

| Anchor | Tension load, N_{ua} (lb) | Shear load x, V_{uax} (lb) | Shear load y, V_{uay} (lb) | Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb) |
|--------|--------------------------------|---------------------------------|---------------------------------|---|
| 1 | 1020.0 | 27.0 | 565.0 | 565.6 |
| Sum | 1020.0 | 27.0 | 565.0 | 565.6 |

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

| N_{sa} (lb) | ϕ | ϕN_{sa} (lb) |
|---------------|--------|--------------------|
| 8095 | 0.75 | 6071 |

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

| k_c | λ | f'_c (psi) | h_{ef} (in) | N_b (lb) |
|-------|-----------|--------------|---------------|------------|
| 17.0 | 1.00 | 2500 | 5.247 | 10215 |

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

| A_{Nc} (in ²) | A_{Nco} (in ²) | $\psi_{ed,N}$ | $\psi_{c,N}$ | $\psi_{cp,N}$ | N_b (lb) | ϕ | ϕN_{cb} (lb) |
|-----------------------------|------------------------------|---------------|--------------|---------------|------------|--------|--------------------|
| 220.36 | 247.75 | 0.967 | 1.00 | 1.000 | 10215 | 0.65 | 5710 |

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

| $\tau_{k,cr}$ (psi) | $f_{short-term}$ | K_{sat} | $\tau_{k,cr}$ (psi) |
|---------------------|------------------|-----------|---------------------|
| 1035 | 1.00 | 1.00 | 1035 |

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

| $\tau_{k,cr}$ (psi) | d_a (in) | h_{ef} (in) | N_{a0} (lb) |
|---------------------|------------|---------------|---------------|
| 1035 | 0.50 | 6.000 | 9755 |

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

| A_{Na} (in ²) | A_{Na0} (in ²) | $\psi_{ed,Na}$ | $\psi_{p,Na}$ | N_{a0} (lb) | ϕ | ϕN_a (lb) |
|-----------------------------|------------------------------|----------------|---------------|---------------|--------|-----------------|
| 109.66 | 109.66 | 1.000 | 1.000 | 9755 | 0.55 | 5365 |

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



| | | | |
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| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

| V_{sa} (lb) | ϕ_{grout} | ϕ | $\phi_{grout}\phi V_{sa}$ (lb) |
|---------------|----------------|--------|--------------------------------|
| 4855 | 1.0 | 0.65 | 3156 |

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f_c (psi) | c_{a1} (in) | V_{by} (lb) |
|------------|------------|-----------|-------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 7.00 | 6947 |

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

| A_{vc} (in ²) | A_{vco} (in ²) | $\psi_{ed,v}$ | $\psi_{c,v}$ | $\psi_{h,v}$ | V_{by} (lb) | ϕ | ϕV_{cbv} (lb) |
|-----------------------------|------------------------------|---------------|--------------|--------------|---------------|--------|---------------------|
| 192.89 | 220.50 | 0.925 | 1.000 | 1.000 | 6947 | 0.70 | 3934 |

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f_c (psi) | c_{a1} (in) | V_{bx} (lb) |
|------------|------------|-----------|-------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 7.87 | 8282 |

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

| A_{vc} (in ²) | A_{vco} (in ²) | $\psi_{ed,v}$ | $\psi_{c,v}$ | $\psi_{h,v}$ | V_{bx} (lb) | ϕ | ϕV_{cbx} (lb) |
|-----------------------------|------------------------------|---------------|--------------|--------------|---------------|--------|---------------------|
| 165.27 | 278.72 | 0.878 | 1.000 | 1.000 | 8282 | 0.70 | 3018 |

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f_c (psi) | c_{a1} (in) | V_{by} (lb) |
|------------|------------|-----------|-------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 7.00 | 6947 |

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

| A_{vc} (in ²) | A_{vco} (in ²) | $\psi_{ed,v}$ | $\psi_{c,v}$ | $\psi_{h,v}$ | V_{by} (lb) | ϕ | ϕV_{cbx} (lb) |
|-----------------------------|------------------------------|---------------|--------------|--------------|---------------|--------|---------------------|
| 192.89 | 220.50 | 1.000 | 1.000 | 1.000 | 6947 | 0.70 | 8508 |

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f_c (psi) | c_{a1} (in) | V_{bx} (lb) |
|------------|------------|-----------|-------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 7.87 | 8282 |

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

| A_{vc} (in ²) | A_{vco} (in ²) | $\psi_{ed,v}$ | $\psi_{c,v}$ | $\psi_{h,v}$ | V_{bx} (lb) | ϕ | ϕV_{cbv} (lb) |
|-----------------------------|------------------------------|---------------|--------------|--------------|---------------|--------|---------------------|
| 165.27 | 278.72 | 1.000 | 1.000 | 1.000 | 8282 | 0.70 | 6875 |

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

| k_{cp} | A_{Na} (in ²) | A_{Na0} (in ²) | $\psi_{ed,Na}$ | $\psi_{p,Na}$ | N_{a0} (lb) | N_a (lb) |
|----------|-----------------------------|------------------------------|----------------|---------------|---------------|------------|
| 2.0 | 109.66 | 109.66 | 1.000 | 1.000 | 9755 | 9755 |

| A_{Nc} (in ²) | A_{Nco} (in ²) | $\psi_{ed,N}$ | $\psi_{c,N}$ | $\psi_{cp,N}$ | N_b (lb) | N_{cb} (lb) | ϕ | ϕV_{cp} (lb) |
|-----------------------------|------------------------------|---------------|--------------|---------------|------------|---------------|--------|--------------------|
| 220.36 | 247.75 | 0.967 | 1.000 | 1.000 | 10215 | 8785 | 0.70 | 12298 |



| | | | |
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| Company: | Schletter, Inc. | Date: | 8/1/2016 |
| Engineer: | HCV | Page: | 5/5 |
| Project: | Standard PVMax - Worst Case, 14-40 Inch Width | | |
| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

| Tension | Factored Load, N _{ua} (lb) | Design Strength, ϕN _n (lb) | Ratio | Status | |
|-----------------------------|-------------------------------------|---------------------------------------|----------------|-----------------------|--------|
| Steel | 1020 | 6071 | 0.17 | Pass | |
| Concrete breakout | 1020 | 5710 | 0.18 | Pass | |
| Adhesive | 1020 | 5365 | 0.19 | Pass (Governs) | |
| | | | | | |
| Shear | Factored Load, V _{ua} (lb) | Design Strength, ϕV _n (lb) | Ratio | Status | |
| Steel | 566 | 3156 | 0.18 | Pass (Governs) | |
| T Concrete breakout y+ | 565 | 3934 | 0.14 | Pass | |
| T Concrete breakout x+ | 27 | 3018 | 0.01 | Pass | |
| Concrete breakout y+ | 27 | 8508 | 0.00 | Pass | |
| Concrete breakout x+ | 565 | 6875 | 0.08 | Pass | |
| Concrete breakout, combined | - | - | 0.14 | Pass | |
| Pryout | 566 | 12298 | 0.05 | Pass | |
| | | | | | |
| Interaction check | N _{ua} /ϕN _n | V _{ua} /ϕV _n | Combined Ratio | Permissible | Status |
| Sec. D.7.1 | 0.19 | 0.00 | 19.0 % | 1.0 | Pass |

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
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| | | | |
|-----------|---|-------|----------|
| Company: | Schletter, Inc. | Date: | 8/1/2016 |
| Engineer: | HCV | Page: | 1/5 |
| Project: | Standard PVMax - Worst Case, 32-40 Inch Width | | |
| Address: | | | |
| Phone: | | | |
| E-mail: | | | |

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

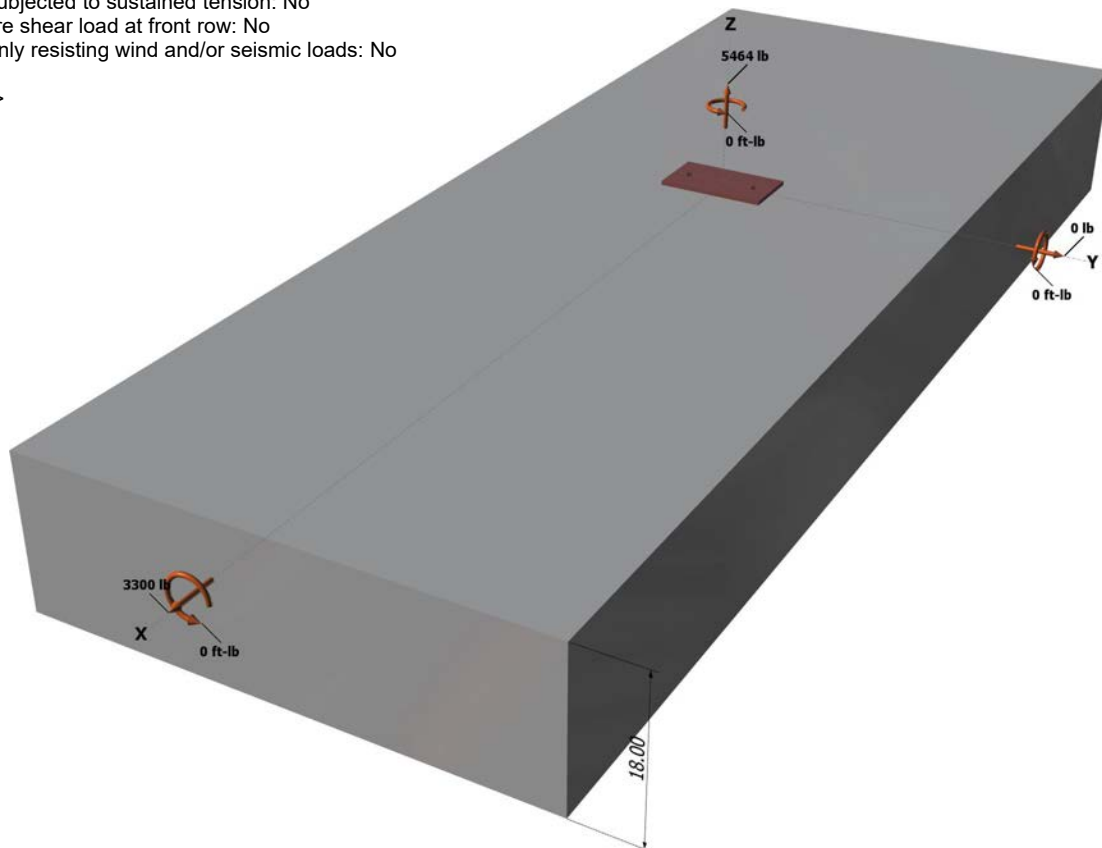
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

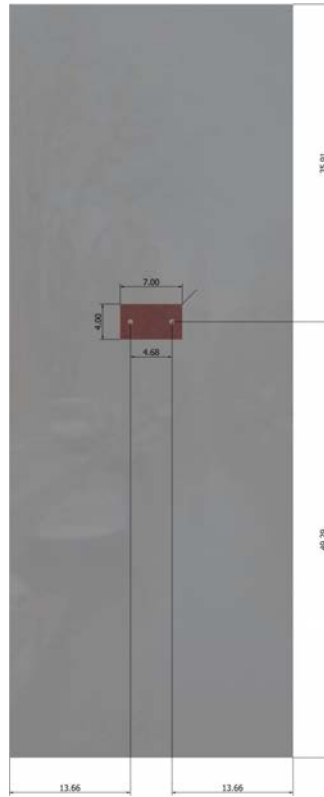
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| Phone: | | | |
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

| Anchor | Tension load, N_{ua} (lb) | Shear load x, V_{uax} (lb) | Shear load y, V_{uay} (lb) | Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb) |
|--------|--------------------------------|---------------------------------|---------------------------------|---|
| 1 | 2732.0 | 1650.0 | 0.0 | 1650.0 |
| 2 | 2732.0 | 1650.0 | 0.0 | 1650.0 |
| Sum | 5464.0 | 3300.0 | 0.0 | 3300.0 |

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

| N_{sa} (lb) | ϕ | ϕN_{sa} (lb) |
|---------------|--------|--------------------|
| 8095 | 0.75 | 6071 |

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

| k_c | λ | f'_c (psi) | h_{ef} (in) | N_b (lb) |
|-------|-----------|--------------|---------------|------------|
| 17.0 | 1.00 | 2500 | 6.000 | 12492 |

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

| A_{Nc} (in ²) | A_{Nco} (in ²) | $\psi_{ec,N}$ | $\psi_{ed,N}$ | $\psi_{c,N}$ | $\psi_{cp,N}$ | N_b (lb) | ϕ | ϕN_{cbg} (lb) |
|-----------------------------|------------------------------|---------------|---------------|--------------|---------------|------------|--------|---------------------|
| 408.24 | 324.00 | 1.000 | 1.000 | 1.00 | 1.000 | 12492 | 0.65 | 10231 |

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

| $\tau_{k,cr}$ (psi) | $f_{\text{short-term}}$ | K_{sat} | $\tau_{k,cr}$ (psi) |
|---------------------|-------------------------|-----------|---------------------|
| 1035 | 1.00 | 1.00 | 1035 |

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

| $\tau_{k,cr}$ (psi) | d_a (in) | h_{ef} (in) | N_{a0} (lb) |
|---------------------|------------|---------------|---------------|
| 1035 | 0.50 | 6.000 | 9755 |

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

| A_{Na} (in ²) | A_{Na0} (in ²) | $\psi_{ed,Na}$ | $\psi_{g,Na}$ | $\psi_{ec,Na}$ | $\psi_{p,Na}$ | N_{a0} (lb) | ϕ | ϕN_{ag} (lb) |
|-----------------------------|------------------------------|----------------|---------------|----------------|---------------|---------------|--------|--------------------|
| 158.66 | 109.66 | 1.000 | 1.043 | 1.000 | 1.000 | 9755 | 0.55 | 8093 |

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

| V_{sa} (lb) | ϕ_{grout} | ϕ | $\phi_{grout}\phi V_{sa}$ (lb) |
|---------------|----------------|--------|--------------------------------|
| 4855 | 1.0 | 0.65 | 3156 |

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f'_c (psi) | c_{a1} (in) | V_{bx} (lb) |
|------------|------------|-----------|--------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 12.00 | 15593 |

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

| A_{Vc} (in ²) | A_{Vco} (in ²) | $\psi_{ec,V}$ | $\psi_{ed,V}$ | $\psi_{c,V}$ | $\psi_{h,V}$ | V_{bx} (lb) | ϕ | ϕV_{cbgx} (lb) |
|-----------------------------|------------------------------|---------------|---------------|--------------|--------------|---------------|--------|----------------------|
| 576.00 | 648.00 | 1.000 | 0.928 | 1.000 | 1.000 | 15593 | 0.70 | 9001 |

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

| l_e (in) | d_a (in) | λ | f'_c (psi) | c_{a1} (in) | V_{by} (lb) |
|------------|------------|-----------|--------------|---------------|---------------|
| 4.00 | 0.50 | 1.00 | 2500 | 13.66 | 18939 |

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

| A_{Vc} (in ²) | A_{Vco} (in ²) | $\psi_{ed,V}$ | $\psi_{c,V}$ | $\psi_{h,V}$ | V_{by} (lb) | ϕ | ϕV_{cbx} (lb) |
|-----------------------------|------------------------------|---------------|--------------|--------------|---------------|--------|---------------------|
| 737.64 | 839.68 | 1.000 | 1.000 | 1.000 | 18939 | 0.70 | 23292 |

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

| k_{cp} | A_{Na} (in ²) | A_{Na0} (in ²) | $\psi_{ed,Na}$ | $\psi_{g,Na}$ | $\psi_{ec,Na}$ | $\psi_{p,Na}$ | N_{a0} (lb) | N_a (lb) |
|----------|-----------------------------|------------------------------|----------------|---------------|----------------|---------------|---------------|------------|
| 2.0 | 158.66 | 109.66 | 1.000 | 1.043 | 1.000 | 1.000 | 9755 | 14715 |

| A_{Nc} (in ²) | A_{Nco} (in ²) | $\psi_{ec,N}$ | $\psi_{ed,N}$ | $\psi_{c,N}$ | $\psi_{cp,N}$ | N_b (lb) | N_{cb} (lb) | ϕ |
|-----------------------------|------------------------------|---------------|---------------|--------------|---------------|------------|---------------|--------|
| 408.24 | 324.00 | 1.000 | 1.000 | 1.000 | 1.000 | 12492 | 15740 | 0.70 |

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

| Tension | Factored Load, N_{ua} (lb) | Design Strength, ϕN_n (lb) | Ratio | Status |
|------------------------|------------------------------|----------------------------------|-------------|-----------------------|
| Steel | 2732 | 6071 | 0.45 | Pass |
| Concrete breakout | 5464 | 10231 | 0.53 | Pass |
| Adhesive | 5464 | 8093 | 0.68 | Pass (Governs) |
| Shear | Factored Load, V_{ua} (lb) | Design Strength, ϕV_n (lb) | Ratio | Status |
| Steel | 1650 | 3156 | 0.52 | Pass (Governs) |
| T Concrete breakout x+ | 3300 | 9001 | 0.37 | Pass |

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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|----------------------|------|-------|------|------|
| Concrete breakout y- | 1650 | 23292 | 0.07 | Pass |
| Pryout | 3300 | 20601 | 0.16 | Pass |

| | | | | | |
|-------------------|-------------------|-------------------|----------------|-------------|--------|
| Interaction check | $N_{ua}/\phi N_n$ | $V_{ua}/\phi V_n$ | Combined Ratio | Permissible | Status |
| Sec. D.7.3 | 0.68 | 0.52 | 119.8 % | 1.2 | Pass |

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.