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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	0.00	$C_s$ = 0	
$S_1$ =	0.00	$\rho$ = 1.3	
$S_{D1}$ =	0.00	$\Omega$ = 1.25	
$T_a$ =	0.00	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

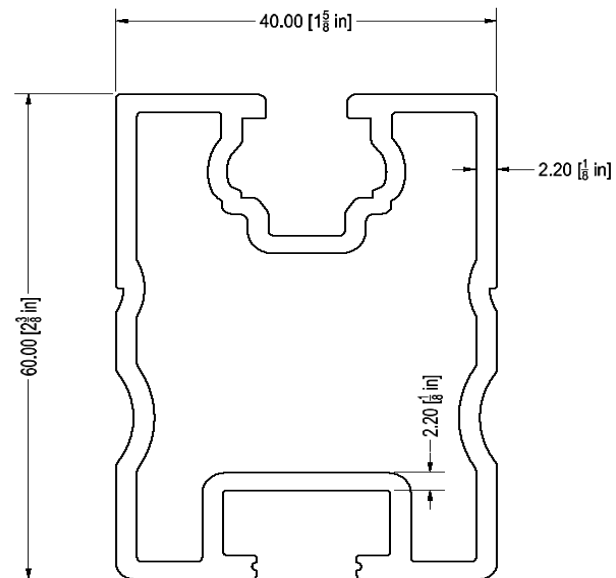
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

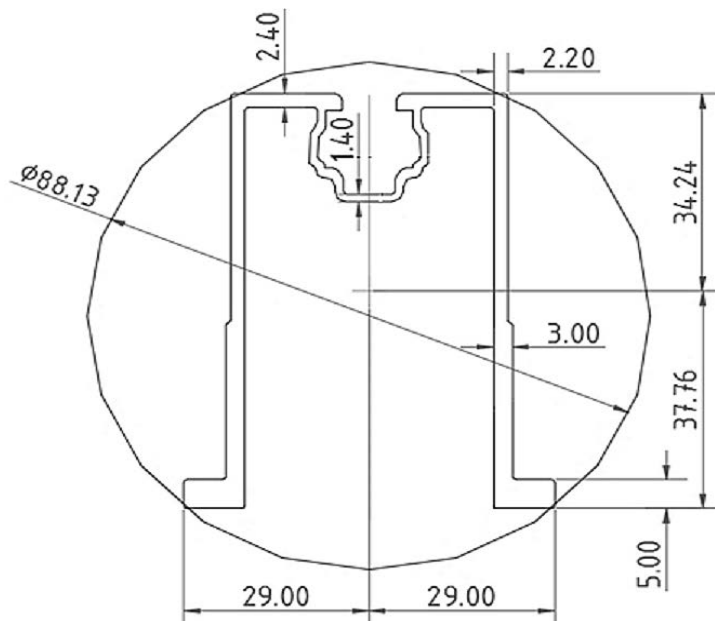
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	54 in
$\Phi F_{ty}$ STRONG-AXIS =	29.52 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.448 k-ft
$M_z$ =	0.060 k-ft
$M_{y \text{ allowable}}$ =	1.256 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>43%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.82 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.489 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.224 k
$M_{y \text{ allowable}}$ =	1.463 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>35%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.129 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>9%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

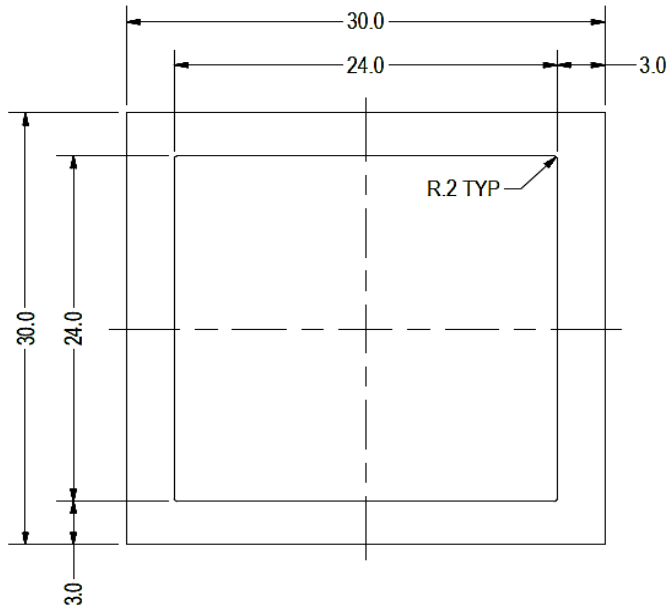
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.184 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>5%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

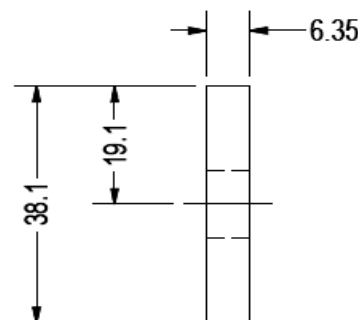
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.863 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>11%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.059 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>5%</b>



A cross brace kit is required every 40 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

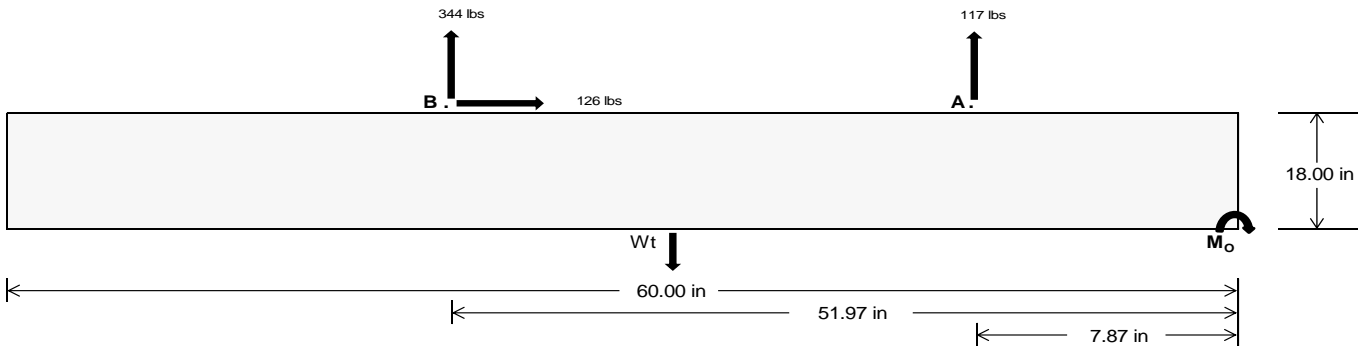
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>513.16</b>	<b>1495.36</b>	k
Compressive Load =	<b>1467.69</b>	<b>1057.89</b>	k
Lateral Load =	<b>1.49</b>	<b>547.40</b>	k
Moment (Weak Axis) =	<b>0.00</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21083.0$  in-lbs  
Resisting Force Required = 702.77 lbs  
S.F. = 1.67  
Weight Required = 1171.28 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 126.30 lbs  
Friction = 0.4  
Weight Required = 315.75 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 126.30 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	471 lbs	471 lbs	471 lbs	471 lbs	548 lbs	548 lbs	548 lbs	548 lbs	731 lbs	731 lbs	731 lbs	731 lbs	-234 lbs	-234 lbs	-234 lbs	-234 lbs
$F_B$	341 lbs	341 lbs	341 lbs	341 lbs	394 lbs	394 lbs	394 lbs	394 lbs	527 lbs	527 lbs	527 lbs	527 lbs	-688 lbs	-688 lbs	-688 lbs	-688 lbs
$F_V$	26 lbs	26 lbs	26 lbs	26 lbs	220 lbs	220 lbs	220 lbs	220 lbs	183 lbs	183 lbs	183 lbs	183 lbs	-253 lbs	-253 lbs	-253 lbs	-253 lbs
$P_{total}$	2716 lbs	2806 lbs	2897 lbs	2988 lbs	2845 lbs	2936 lbs	3026 lbs	3117 lbs	3161 lbs	3252 lbs	3343 lbs	3433 lbs	219 lbs	274 lbs	328 lbs	382 lbs
$M$	283 lbs-ft	283 lbs-ft	283 lbs-ft	283 lbs-ft	620 lbs-ft	620 lbs-ft	620 lbs-ft	620 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	449 lbs-ft	449 lbs-ft	449 lbs-ft	449 lbs-ft
$e$	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.22 ft	0.21 ft	0.20 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.20 ft	0.19 ft	2.05 ft	1.64 ft	1.37 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	271.5 psf	269.1 psf	266.8 psf	264.8 psf	240.1 psf	239.1 psf	238.2 psf	237.3 psf	270.9 psf	268.4 psf	266.2 psf	264.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	349.2 psf	343.2 psf	337.7 psf	332.7 psf	410.1 psf	401.4 psf	393.4 psf	386.0 psf	451.7 psf	441.1 psf	431.4 psf	422.4 psf	185.5 psf	116.0 psf	101.0 psf	96.2 psf

Maximum Bearing Pressure = 452 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

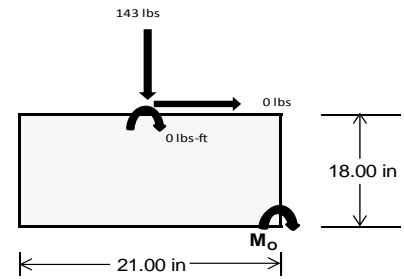
## Overturning Check

$M_o = 0.0 \text{ ft-lbs}$   
 Resisting Force Required = 0.00 lbs  
 S.F. = 1.67  
 Weight Required = 0.00 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	55 lbs	143 lbs	52 lbs	218 lbs	659 lbs	215 lbs	16 lbs	42 lbs	15 lbs
$F_v$	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2411 lbs	2499 lbs	2408 lbs	2461 lbs	2902 lbs	2458 lbs	705 lbs	731 lbs	704 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
$f_{min}$	275.5 sqft	285.6 sqft	275.2 sqft	280.9 sqft	331.4 sqft	280.8 sqft	80.5 sqft	83.5 sqft	80.5 sqft
$f_{max}$	275.6 psf	285.6 psf	275.2 psf	281.5 psf	331.8 psf	281.0 psf	80.6 psf	83.5 psf	80.5 psf



Maximum Bearing Pressure = 332 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.**

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



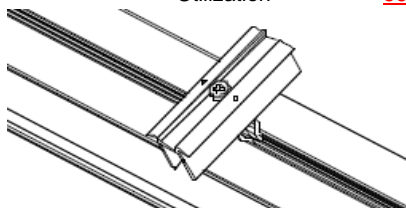
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

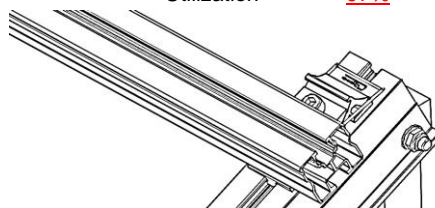
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.677 k
Allowable Uplift =	1.214 k
Utilization =	<u>56%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.083 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



### 6.2 Bolted Connections

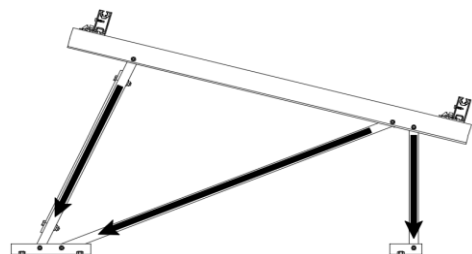
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.129 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Diagonal Strut

Maximum Axial Load =	0.184 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.138 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.059 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.005 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * L_b / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	110.188	2	241.406	2	.016	9	0	9	0	1	0	1
2		min	-137.974	3	-355.843	3	-.173	3	0	3	0	1	0	1
3	N7	max	0	15	374.267	1	0	10	0	10	0	1	0	1
4		min	-.109	2	-115.292	3	-.421	1	0	1	0	1	0	1
5	N15	max	0	15	1128.996	1	.134	9	0	1	0	1	0	1
6		min	-1.145	2	-394.736	3	-.408	3	0	3	0	1	0	1
7	N16	max	377.643	2	813.765	1	0	10	0	9	0	1	0	1
8		min	-421.078	3	-1150.274	3	-50.346	3	0	3	0	1	0	1
9	N23	max	0	15	374.393	1	.733	1	.001	1	0	1	0	1
10		min	-.109	2	-114.936	3	.001	10	0	10	0	1	0	1
11	N24	max	110.189	2	244.132	1	50.794	3	0	1	0	1	0	1
12		min	-138.215	3	-354.529	3	-.002	10	0	3	0	1	0	1
13	Totals:	max	596.657	2	3176.787	1	0	12						
14		min	-697.628	3	-2485.611	3	0	1						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	269.54	1	.668	4	.2	1	0	10	0	3	0	1
2			min	-357.634	3	.158	15	-.121	3	0	1	0	2	0	1
3		2	max	269.636	1	.63	4	.2	1	0	10	0	1	0	15
4			min	-357.562	3	.149	15	-.121	3	0	1	0	10	0	4
5		3	max	269.733	1	.592	4	.2	1	0	10	0	1	0	15
6			min	-357.49	3	.14	15	-.121	3	0	1	0	3	0	4
7		4	max	269.829	1	.554	4	.2	1	0	10	0	1	0	15
8			min	-357.418	3	.131	15	-.121	3	0	1	0	3	0	4
9		5	max	269.925	1	.516	4	.2	1	0	10	0	1	0	15
10			min	-357.345	3	.122	15	-.121	3	0	1	0	3	0	4
11		6	max	270.022	1	.479	4	.2	1	0	10	0	1	0	15
12			min	-357.273	3	.113	15	-.121	3	0	1	0	3	0	4
13		7	max	270.118	1	.441	4	.2	1	0	10	0	1	0	15
14			min	-357.201	3	.105	15	-.121	3	0	1	0	3	0	4
15		8	max	270.214	1	.403	4	.2	1	0	10	0	1	0	15
16			min	-357.129	3	.096	15	-.121	3	0	1	0	3	0	4
17		9	max	270.311	1	.365	4	.2	1	0	10	0	1	0	15
18			min	-357.056	3	.087	15	-.121	3	0	1	0	3	0	4
19		10	max	270.407	1	.327	4	.2	1	0	10	0	1	0	15
20			min	-356.984	3	.078	15	-.121	3	0	1	0	3	0	4
21		11	max	270.504	1	.289	4	.2	1	0	10	0	1	0	15
22			min	-356.912	3	.069	15	-.121	3	0	1	0	3	0	4
23		12	max	270.6	1	.252	4	.2	1	0	10	0	1	0	15
24			min	-356.839	3	.06	15	-.121	3	0	1	0	3	0	4
25		13	max	270.696	1	.214	4	.2	1	0	10	0	1	0	15
26			min	-356.767	3	.051	15	-.121	3	0	1	0	3	0	4
27		14	max	270.793	1	.176	4	.2	1	0	10	0	1	0	15
28			min	-356.695	3	.042	15	-.121	3	0	1	0	3	0	4
29		15	max	270.889	1	.138	4	.2	1	0	10	0	1	0	15
30			min	-356.623	3	.033	15	-.121	3	0	1	0	3	0	4
31		16	max	270.985	1	.1	4	.2	1	0	10	0	1	0	15
32			min	-356.55	3	.024	15	-.121	3	0	1	0	3	0	4
33		17	max	271.082	1	.064	2	.2	1	0	10	0	1	0	15
34			min	-356.478	3	.016	15	-.121	3	0	1	0	3	0	4
35		18	max	271.178	1	.034	2	.2	1	0	10	0	1	0	15
36			min	-356.406	3	0	9	-.121	3	0	1	0	3	0	4
37		19	max	271.274	1	.007	10	.2	1	0	10	0	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-356.333	3	-.027	1	-.121	3	0	1	0	3	0	4
39	M3	1	max	48.764	2	1.816	4	-.003	10	0	10	0	1	4
40		min	-55.49	9	.428	15	-.186	1	0	1	0	10	0	15
41		2	max	48.697	2	1.638	4	-.003	10	0	10	0	1	4
42		min	-55.546	9	.386	15	-.186	1	0	1	0	10	0	15
43		3	max	48.63	2	1.46	4	-.003	10	0	10	0	1	2
44		min	-55.602	9	.344	15	-.186	1	0	1	0	10	0	15
45		4	max	48.563	2	1.282	4	-.003	10	0	10	0	1	15
46		min	-55.658	9	.302	15	-.186	1	0	1	0	10	0	4
47		5	max	48.496	2	1.104	4	-.003	10	0	10	0	1	15
48		min	-55.714	9	.26	15	-.186	1	0	1	0	10	0	4
49		6	max	48.429	2	.926	4	-.003	10	0	10	0	1	15
50		min	-55.77	9	.218	15	-.186	1	0	1	0	10	0	4
51		7	max	48.362	2	.748	4	-.003	10	0	10	0	1	15
52		min	-55.826	9	.176	15	-.186	1	0	1	0	10	0	4
53		8	max	48.294	2	.57	4	-.003	10	0	10	0	1	15
54		min	-55.882	9	.135	15	-.186	1	0	1	0	10	0	4
55		9	max	48.227	2	.392	4	-.003	10	0	10	0	1	15
56		min	-55.938	9	.093	15	-.186	1	0	1	0	10	-.001	4
57		10	max	48.16	2	.214	4	-.003	10	0	10	0	1	15
58		min	-55.994	9	.051	15	-.186	1	0	1	0	10	-.001	4
59		11	max	48.093	2	.038	2	-.003	10	0	10	0	1	15
60		min	-56.049	9	.009	15	-.186	1	0	1	0	10	-.001	4
61		12	max	48.026	2	-.033	15	-.003	10	0	10	0	1	15
62		min	-56.105	9	-.142	4	-.186	1	0	1	0	10	-.001	4
63		13	max	47.959	2	-.075	15	-.003	10	0	10	0	1	15
64		min	-56.161	9	-.32	4	-.186	1	0	1	0	10	-.001	4
65		14	max	47.892	2	-.116	15	-.003	10	0	10	0	9	15
66		min	-56.217	9	-.498	4	-.186	1	0	1	0	2	-.001	4
67		15	max	47.825	2	-.158	15	-.003	10	0	10	0	10	15
68		min	-56.273	9	-.676	4	-.186	1	0	1	0	1	0	4
69		16	max	47.758	2	-.2	15	-.003	10	0	10	0	10	15
70		min	-56.329	9	-.854	4	-.186	1	0	1	0	1	0	4
71		17	max	47.691	2	-.242	15	-.003	10	0	10	0	10	15
72		min	-56.385	9	-1.032	4	-.186	1	0	1	0	1	0	4
73		18	max	47.623	2	-.284	15	-.003	10	0	10	0	10	15
74		min	-56.441	9	-1.21	4	-.186	1	0	1	0	1	0	4
75		19	max	47.556	2	-.326	15	-.003	10	0	10	0	10	1
76		min	-56.497	9	-1.388	4	-.186	1	0	1	0	1	0	1
77	M4	1	max	373.102	1	0	1	-.001	10	0	1	0	3	1
78		min	-116.166	3	0	1	-.449	1	0	1	0	2	0	1
79		2	max	373.167	1	0	1	-.001	10	0	1	0	15	1
80		min	-116.117	3	0	1	-.449	1	0	1	0	1	0	1
81		3	max	373.231	1	0	1	-.001	10	0	1	0	15	1
82		min	-116.068	3	0	1	-.449	1	0	1	0	1	0	1
83		4	max	373.296	1	0	1	-.001	10	0	1	0	15	1
84		min	-116.02	3	0	1	-.449	1	0	1	0	1	0	1
85		5	max	373.361	1	0	1	-.001	10	0	1	0	15	1
86		min	-115.971	3	0	1	-.449	1	0	1	0	1	0	1
87		6	max	373.426	1	0	1	-.001	10	0	1	0	15	1
88		min	-115.923	3	0	1	-.449	1	0	1	0	1	0	1
89		7	max	373.49	1	0	1	-.001	10	0	1	0	15	1
90		min	-115.874	3	0	1	-.449	1	0	1	0	1	0	1
91		8	max	373.555	1	0	1	-.001	10	0	1	0	10	1
92		min	-115.826	3	0	1	-.449	1	0	1	0	1	0	1
93		9	max	373.62	1	0	1	-.001	10	0	1	0	10	1
94		min	-115.777	3	0	1	-.449	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	373.684	1	0	1	-.001	10	0	1	0	10	0	1
96			min	-115.729	3	0	1	-.449	1	0	1	0	1	0	1
97		11	max	373.749	1	0	1	-.001	10	0	1	0	10	0	1
98			min	-115.68	3	0	1	-.449	1	0	1	0	1	0	1
99		12	max	373.814	1	0	1	-.001	10	0	1	0	10	0	1
100			min	-115.632	3	0	1	-.449	1	0	1	0	1	0	1
101		13	max	373.879	1	0	1	-.001	10	0	1	0	10	0	1
102			min	-115.583	3	0	1	-.449	1	0	1	0	1	0	1
103		14	max	373.943	1	0	1	-.001	10	0	1	0	10	0	1
104			min	-115.535	3	0	1	-.449	1	0	1	0	1	0	1
105		15	max	374.008	1	0	1	-.001	10	0	1	0	10	0	1
106			min	-115.486	3	0	1	-.449	1	0	1	0	1	0	1
107		16	max	374.073	1	0	1	-.001	10	0	1	0	10	0	1
108			min	-115.438	3	0	1	-.449	1	0	1	0	1	0	1
109		17	max	374.137	1	0	1	-.001	10	0	1	0	10	0	1
110			min	-115.389	3	0	1	-.449	1	0	1	0	1	0	1
111		18	max	374.202	1	0	1	-.001	10	0	1	0	10	0	1
112			min	-115.341	3	0	1	-.449	1	0	1	0	1	0	1
113		19	max	374.267	1	0	1	-.001	10	0	1	0	10	0	1
114			min	-115.292	3	0	1	-.449	1	0	1	0	1	0	1
115	M6	1	max	861.423	1	.656	4	.055	9	0	3	0	3	0	1
116			min	-1138.46	3	.156	15	-.25	3	0	2	0	1	0	1
117		2	max	861.519	1	.618	4	.055	9	0	3	0	3	0	15
118			min	-1138.388	3	.147	15	-.25	3	0	2	0	2	0	4
119		3	max	861.616	1	.581	4	.055	9	0	3	0	3	0	15
120			min	-1138.316	3	.138	15	-.25	3	0	2	0	2	0	4
121		4	max	861.712	1	.543	4	.055	9	0	3	0	9	0	15
122			min	-1138.243	3	.13	15	-.25	3	0	2	0	2	0	4
123		5	max	861.809	1	.505	4	.055	9	0	3	0	9	0	15
124			min	-1138.171	3	.121	15	-.25	3	0	2	0	3	0	4
125		6	max	861.905	1	.467	4	.055	9	0	3	0	9	0	15
126			min	-1138.099	3	.112	15	-.25	3	0	2	0	3	0	4
127		7	max	862.001	1	.429	4	.055	9	0	3	0	9	0	15
128			min	-1138.027	3	.103	15	-.25	3	0	2	0	3	0	4
129		8	max	862.098	1	.391	4	.055	9	0	3	0	9	0	15
130			min	-1137.954	3	.094	15	-.25	3	0	2	0	3	0	4
131		9	max	862.194	1	.354	4	.055	9	0	3	0	9	0	15
132			min	-1137.882	3	.085	15	-.25	3	0	2	0	3	0	4
133		10	max	862.29	1	.316	4	.055	9	0	3	0	9	0	15
134			min	-1137.81	3	.076	15	-.25	3	0	2	0	3	0	4
135		11	max	862.387	1	.278	4	.055	9	0	3	0	9	0	15
136			min	-1137.737	3	.067	15	-.25	3	0	2	0	3	0	4
137		12	max	862.483	1	.24	4	.055	9	0	3	0	9	0	15
138			min	-1137.665	3	.058	15	-.25	3	0	2	0	3	0	4
139		13	max	862.579	1	.21	2	.055	9	0	3	0	9	0	15
140			min	-1137.593	3	.05	15	-.25	3	0	2	0	3	0	4
141		14	max	862.676	1	.18	2	.055	9	0	3	0	9	0	15
142			min	-1137.521	3	.041	15	-.25	3	0	2	0	3	0	4
143		15	max	862.772	1	.151	2	.055	9	0	3	0	9	0	15
144			min	-1137.448	3	.032	15	-.25	3	0	2	0	3	0	4
145		16	max	862.869	1	.121	2	.055	9	0	3	0	9	0	15
146			min	-1137.376	3	.021	9	-.25	3	0	2	0	3	0	4
147		17	max	862.965	1	.092	2	.055	9	0	3	0	9	0	15
148			min	-1137.304	3	-.003	9	-.25	3	0	2	0	3	0	4
149		18	max	863.061	1	.062	2	.055	9	0	3	0	9	0	15
150			min	-1137.232	3	-.028	9	-.25	3	0	2	0	3	0	4
151		19	max	863.158	1	.033	2	.055	9	0	3	0	9	0	15





Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1137.159	3	-.052	9	-.25	3	0	2	0	3	0	4
153		max	184.006	2	1.812	4	0	13	0	1	0	1	0	4
154		min	-100.03	9	.427	15	-.012	1	0	3	0	3	0	15
155		max	183.939	2	1.634	4	0	13	0	1	0	1	0	2
156		min	-100.085	9	.385	15	-.012	1	0	3	0	3	0	15
157		max	183.872	2	1.456	4	0	13	0	1	0	1	0	2
158		min	-100.141	9	.343	15	-.012	1	0	3	0	3	0	9
159		max	183.805	2	1.278	4	0	13	0	1	0	1	0	10
160		min	-100.197	9	.301	15	-.012	1	0	3	0	3	0	9
161		max	183.738	2	1.1	4	0	13	0	1	0	1	0	15
162	M8	min	-100.253	9	.26	15	-.012	1	0	3	0	3	0	4
163		max	183.67	2	.922	4	0	13	0	1	0	1	0	15
164		min	-100.309	9	.218	15	-.012	1	0	3	0	3	0	4
165		max	183.603	2	.743	4	0	13	0	1	0	1	0	15
166		min	-100.365	9	.176	15	-.012	1	0	3	0	3	0	4
167		max	183.536	2	.565	4	0	13	0	1	0	1	0	15
168		min	-100.421	9	.134	15	-.012	1	0	3	0	3	0	4
169		max	183.469	2	.387	4	0	13	0	1	0	1	0	15
170		min	-100.477	9	.092	15	-.012	1	0	3	0	3	-.001	4
171		max	183.402	2	.209	4	0	13	0	1	0	1	0	15
172	M9	min	-100.533	9	.05	15	-.012	1	0	3	0	3	-.001	4
173		max	183.335	2	.056	2	0	13	0	1	0	1	0	15
174		min	-100.589	9	-.001	9	-.012	1	0	3	0	3	-.001	4
175		max	183.268	2	-.033	15	0	13	0	1	0	1	0	15
176		min	-100.645	9	-.147	4	-.012	1	0	3	0	3	-.001	4
177		max	183.201	2	-.075	15	0	13	0	1	0	1	0	15
178		min	-100.7	9	-.325	4	-.012	1	0	3	0	3	-.001	4
179		max	183.134	2	-.117	15	0	13	0	1	0	1	0	15
180		min	-100.756	9	-.503	4	-.012	1	0	3	0	3	-.001	4
181		max	183.067	2	-.159	15	0	13	0	1	0	1	0	15
182	M10	min	-100.812	9	-.681	4	-.012	1	0	3	0	3	0	4
183		max	182.999	2	-.201	15	0	13	0	1	0	1	0	15
184		min	-100.868	9	-.859	4	-.012	1	0	3	0	3	0	4
185		max	182.932	2	-.243	15	0	13	0	1	0	1	0	15
186		min	-100.924	9	-1.037	4	-.012	1	0	3	0	3	0	4
187		max	182.865	2	-.284	15	0	13	0	1	0	1	0	15
188		min	-100.98	9	-1.215	4	-.012	1	0	3	0	3	0	4
189		max	182.798	2	-.326	15	0	13	0	1	0	1	0	1
190		min	-101.036	9	-1.393	4	-.012	1	0	3	0	3	0	1
191		max	1127.831	1	0	1	.163	1	0	1	0	2	0	1
192	M11	min	-395.61	3	0	1	-.384	3	0	1	0	1	0	1
193		max	1127.896	1	0	1	.163	1	0	1	0	1	0	1
194		min	-395.562	3	0	1	-.384	3	0	1	0	3	0	1
195		max	1127.961	1	0	1	.163	1	0	1	0	1	0	1
196		min	-395.513	3	0	1	-.384	3	0	1	0	3	0	1
197		max	1128.025	1	0	1	.163	1	0	1	0	1	0	1
198		min	-395.464	3	0	1	-.384	3	0	1	0	3	0	1
199		max	1128.09	1	0	1	.163	1	0	1	0	1	0	1
200		min	-395.416	3	0	1	-.384	3	0	1	0	3	0	1
201		max	1128.155	1	0	1	.163	1	0	1	0	1	0	1
202	M12	min	-395.367	3	0	1	-.384	3	0	1	0	3	0	1
203		max	1128.219	1	0	1	.163	1	0	1	0	1	0	1
204		min	-395.319	3	0	1	-.384	3	0	1	0	3	0	1
205		max	1128.284	1	0	1	.163	1	0	1	0	1	0	1
206		min	-395.27	3	0	1	-.384	3	0	1	0	3	0	1
207		max	1128.349	1	0	1	.163	1	0	1	0	1	0	1
208		min	-395.222	3	0	1	-.384	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1128.414	1	0	1	.163	1	0	1	0	1	0	1
210			min	-395.173	3	0	1	-.384	3	0	1	0	3	0	1
211		11	max	1128.478	1	0	1	.163	1	0	1	0	1	0	1
212			min	-395.125	3	0	1	-.384	3	0	1	0	3	0	1
213		12	max	1128.543	1	0	1	.163	1	0	1	0	1	0	1
214			min	-395.076	3	0	1	-.384	3	0	1	0	3	0	1
215		13	max	1128.608	1	0	1	.163	1	0	1	0	1	0	1
216			min	-395.028	3	0	1	-.384	3	0	1	0	3	0	1
217		14	max	1128.672	1	0	1	.163	1	0	1	0	1	0	1
218			min	-394.979	3	0	1	-.384	3	0	1	0	3	0	1
219		15	max	1128.737	1	0	1	.163	1	0	1	0	1	0	1
220			min	-394.931	3	0	1	-.384	3	0	1	0	3	0	1
221		16	max	1128.802	1	0	1	.163	1	0	1	0	1	0	1
222			min	-394.882	3	0	1	-.384	3	0	1	0	3	0	1
223		17	max	1128.867	1	0	1	.163	1	0	1	0	1	0	1
224			min	-394.834	3	0	1	-.384	3	0	1	0	3	0	1
225		18	max	1128.931	1	0	1	.163	1	0	1	0	1	0	1
226			min	-394.785	3	0	1	-.384	3	0	1	0	3	0	1
227		19	max	1128.996	1	0	1	.163	1	0	1	0	1	0	1
228			min	-394.736	3	0	1	-.384	3	0	1	0	3	0	1
229	M10	1	max	271.176	1	.668	4	-.002	15	0	1	0	1	0	1
230			min	-328.677	3	.158	15	-.097	1	0	3	0	3	0	1
231		2	max	271.272	1	.63	4	-.002	15	0	1	0	1	0	15
232			min	-328.605	3	.149	15	-.097	1	0	3	0	3	0	4
233		3	max	271.368	1	.592	4	-.002	15	0	1	0	1	0	15
234			min	-328.533	3	.14	15	-.097	1	0	3	0	3	0	4
235		4	max	271.465	1	.554	4	-.002	15	0	1	0	10	0	15
236			min	-328.461	3	.131	15	-.097	1	0	3	0	3	0	4
237		5	max	271.561	1	.516	4	-.002	15	0	1	0	10	0	15
238			min	-328.388	3	.122	15	-.097	1	0	3	0	3	0	4
239		6	max	271.658	1	.478	4	-.002	15	0	1	0	10	0	15
240			min	-328.316	3	.113	15	-.097	1	0	3	0	3	0	4
241		7	max	271.754	1	.441	4	-.002	15	0	1	0	10	0	15
242			min	-328.244	3	.104	15	-.097	1	0	3	0	3	0	4
243		8	max	271.85	1	.403	4	-.002	15	0	1	0	10	0	15
244			min	-328.171	3	.096	15	-.097	1	0	3	0	3	0	4
245		9	max	271.947	1	.365	4	-.002	15	0	1	0	10	0	15
246			min	-328.099	3	.087	15	-.097	1	0	3	0	3	0	4
247		10	max	272.043	1	.327	4	-.002	15	0	1	0	10	0	15
248			min	-328.027	3	.078	15	-.097	1	0	3	0	3	0	4
249		11	max	272.139	1	.289	4	-.002	15	0	1	0	10	0	15
250			min	-327.955	3	.069	15	-.097	1	0	3	0	3	0	4
251		12	max	272.236	1	.251	4	-.002	15	0	1	0	15	0	15
252			min	-327.882	3	.06	15	-.097	1	0	3	0	3	0	4
253		13	max	272.332	1	.214	4	-.002	15	0	1	0	15	0	15
254			min	-327.81	3	.051	15	-.097	1	0	3	0	3	0	4
255		14	max	272.428	1	.176	4	-.002	15	0	1	0	15	0	15
256			min	-327.738	3	.042	15	-.097	1	0	3	0	3	0	4
257		15	max	272.525	1	.138	4	-.002	15	0	1	0	15	0	15
258			min	-327.666	3	.033	15	-.097	1	0	3	0	3	0	4
259		16	max	272.621	1	.1	4	-.002	15	0	1	0	15	0	15
260			min	-327.593	3	.024	15	-.097	1	0	3	0	3	0	4
261		17	max	272.718	1	.077	3	-.002	15	0	1	0	15	0	15
262			min	-327.521	3	.016	15	-.097	1	0	3	0	3	0	4
263		18	max	272.814	1	.055	3	-.002	15	0	1	0	15	0	15
264			min	-327.449	3	-.001	9	-.097	1	0	3	0	3	0	4
265		19	max	272.91	1	.033	3	-.002	15	0	1	0	15	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC	
266	M11	1	min	-327.376	3	-.028	1	-.097	1	0	3	0	3	0	4	
267		1	max	48.321	2	1.816	4	.203	1	0	3	0	3	0	4	
268			min	-55.573	9	.427	15	-.017	3	0	10	0	1	0	15	
269		2	max	48.254	2	1.638	4	.203	1	0	3	0	3	0	4	
270			min	-55.629	9	.386	15	-.017	3	0	10	0	1	0	15	
271		3	max	48.187	2	1.46	4	.203	1	0	3	0	3	0	2	
272			min	-55.685	9	.344	15	-.017	3	0	10	0	1	0	3	
273		4	max	48.12	2	1.282	4	.203	1	0	3	0	3	0	15	
274			min	-55.741	9	.302	15	-.017	3	0	10	0	1	0	4	
275		5	max	48.052	2	1.104	4	.203	1	0	3	0	3	0	15	
276			min	-55.797	9	.26	15	-.017	3	0	10	0	1	0	4	
277		6	max	47.985	2	.926	4	.203	1	0	3	0	3	0	15	
278			min	-55.853	9	.218	15	-.017	3	0	10	0	1	0	4	
279		7	max	47.918	2	.748	4	.203	1	0	3	0	3	0	15	
280			min	-55.909	9	.176	15	-.017	3	0	10	0	1	0	4	
281		8	max	47.851	2	.57	4	.203	1	0	3	0	3	0	15	
282			min	-55.965	9	.135	15	-.017	3	0	10	0	1	0	4	
283		9	max	47.784	2	.392	4	.203	1	0	3	0	3	0	15	
284			min	-56.021	9	.093	15	-.017	3	0	10	0	1	-.001	4	
285		10	max	47.717	2	.214	4	.203	1	0	3	0	3	0	15	
286			min	-56.077	9	.051	15	-.017	3	0	10	0	1	-.001	4	
287		11	max	47.65	2	.038	2	.203	1	0	3	0	3	0	15	
288			min	-56.133	9	.002	3	-.017	3	0	10	0	1	-.001	4	
289		12	max	47.583	2	-.033	15	.203	1	0	3	0	3	0	15	
290			min	-56.188	9	-.142	4	-.017	3	0	10	0	1	-.001	4	
291		13	max	47.516	2	-.075	15	.203	1	0	3	0	3	0	15	
292			min	-56.244	9	-.32	4	-.017	3	0	10	0	2	-.001	4	
293		14	max	47.449	2	-.117	15	.203	1	0	3	0	3	0	15	
294			min	-56.3	9	-.498	4	-.017	3	0	10	0	10	-.001	4	
295		15	max	47.381	2	-.158	15	.203	1	0	3	0	3	0	15	
296			min	-56.356	9	-.676	4	-.017	3	0	10	0	10	0	4	
297		16	max	47.314	2	-.2	15	.203	1	0	3	0	3	0	15	
298			min	-56.412	9	-.854	4	-.017	3	0	10	0	10	0	4	
299		17	max	47.247	2	-.242	15	.203	1	0	3	0	3	0	15	
300			min	-56.468	9	-1.032	4	-.017	3	0	10	0	10	0	4	
301		18	max	47.18	2	-.284	15	.203	1	0	3	0	3	0	15	
302			min	-56.524	9	-1.21	4	-.017	3	0	10	0	10	0	4	
303		19	max	47.113	2	-.326	15	.203	1	0	3	0	3	0	1	
304			min	-56.58	9	-1.388	4	-.017	3	0	10	0	10	0	1	
305		M12	1	max	373.229	1	0	1	.781	1	0	1	0	2	0	1
306			min	-115.809	3	0	1	.001	10	0	1	0	3	0	1	
307		2	max	373.293	1	0	1	.781	1	0	1	0	1	0	1	
308			min	-115.761	3	0	1	.001	10	0	1	0	15	0	1	
309		3	max	373.358	1	0	1	.781	1	0	1	0	1	0	1	
310			min	-115.712	3	0	1	.001	10	0	1	0	15	0	1	
311		4	max	373.423	1	0	1	.781	1	0	1	0	1	0	1	
312			min	-115.664	3	0	1	.001	10	0	1	0	15	0	1	
313		5	max	373.488	1	0	1	.781	1	0	1	0	1	0	1	
314			min	-115.615	3	0	1	.001	10	0	1	0	10	0	1	
315		6	max	373.552	1	0	1	.781	1	0	1	0	1	0	1	
316			min	-115.567	3	0	1	.001	10	0	1	0	10	0	1	
317		7	max	373.617	1	0	1	.781	1	0	1	0	1	0	1	
318			min	-115.518	3	0	1	.001	10	0	1	0	10	0	1	
319		8	max	373.682	1	0	1	.781	1	0	1	0	1	0	1	
320			min	-115.47	3	0	1	.001	10	0	1	0	10	0	1	
321		9	max	373.746	1	0	1	.781	1	0	1	0	1	0	1	
322			min	-115.421	3	0	1	.001	10	0	1	0	10	0	1	



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	373.811	1	0	1	.781	1	0	1	0	1	0	1
324		min	-115.373	3	0	1	.001	10	0	1	0	10	0	1
325	11	max	373.876	1	0	1	.781	1	0	1	0	1	0	1
326		min	-115.324	3	0	1	.001	10	0	1	0	10	0	1
327	12	max	373.941	1	0	1	.781	1	0	1	0	1	0	1
328		min	-115.275	3	0	1	.001	10	0	1	0	10	0	1
329	13	max	374.005	1	0	1	.781	1	0	1	0	1	0	1
330		min	-115.227	3	0	1	.001	10	0	1	0	10	0	1
331	14	max	374.07	1	0	1	.781	1	0	1	0	1	0	1
332		min	-115.178	3	0	1	.001	10	0	1	0	10	0	1
333	15	max	374.135	1	0	1	.781	1	0	1	0	1	0	1
334		min	-115.13	3	0	1	.001	10	0	1	0	10	0	1
335	16	max	374.199	1	0	1	.781	1	0	1	.001	1	0	1
336		min	-115.081	3	0	1	.001	10	0	1	0	10	0	1
337	17	max	374.264	1	0	1	.781	1	0	1	.001	1	0	1
338		min	-115.033	3	0	1	.001	10	0	1	0	10	0	1
339	18	max	374.329	1	0	1	.781	1	0	1	.001	1	0	1
340		min	-114.984	3	0	1	.001	10	0	1	0	10	0	1
341	19	max	374.393	1	0	1	.781	1	0	1	.001	1	0	1
342		min	-114.936	3	0	1	.001	10	0	1	0	10	0	1
343	M1	1	max	59.741	1	336.525	3	-.261	10	0	.033	1	0	1
344		min	1.846	15	-271.899	1	-17.02	1	0	3	0	10	0	3
345	2	max	59.813	1	336.323	3	-.261	10	0	1	.03	1	.059	1
346		min	1.868	15	-272.169	1	-17.02	1	0	3	0	10	-.073	3
347	3	max	69.799	1	4.418	9	-.257	10	0	3	.026	1	.117	1
348		min	-6.422	3	-21.327	3	-16.881	1	0	1	0	10	-.145	3
349	4	max	69.872	1	4.193	9	-.257	10	0	3	.022	1	.118	1
350		min	-6.368	3	-21.529	3	-16.881	1	0	1	0	10	-.14	3
351	5	max	69.944	1	3.968	9	-.257	10	0	3	.018	1	.119	1
352		min	-6.314	3	-21.732	3	-16.881	1	0	1	0	10	-.135	3
353	6	max	70.016	1	3.744	9	-.257	10	0	3	.015	1	.119	1
354		min	-6.26	3	-21.934	3	-16.881	1	0	1	0	10	-.131	3
355	7	max	70.088	1	3.519	9	-.257	10	0	3	.011	1	.12	1
356		min	-6.206	3	-22.136	3	-16.881	1	0	1	0	10	-.126	3
357	8	max	70.161	1	3.294	9	-.257	10	0	3	.007	1	.121	1
358		min	-6.151	3	-22.339	3	-16.881	1	0	1	0	10	-.121	3
359	9	max	70.233	1	3.069	9	-.257	10	0	3	.004	1	.124	2
360		min	-6.097	3	-22.541	3	-16.881	1	0	1	0	10	-.116	3
361	10	max	70.305	1	2.844	9	-.257	10	0	3	.001	3	.128	2
362		min	-6.043	3	-22.743	3	-16.881	1	0	1	0	15	-.111	3
363	11	max	70.378	1	2.62	9	-.257	10	0	3	0	3	.131	2
364		min	-5.989	3	-22.945	3	-16.881	1	0	1	-.004	1	-.106	3
365	12	max	70.45	1	2.395	9	-.257	10	0	3	0	12	.135	2
366		min	-5.935	3	-23.148	3	-16.881	1	0	1	-.007	1	-.101	3
367	13	max	70.522	1	2.17	9	-.257	10	0	3	0	10	.139	2
368		min	-5.88	3	-23.35	3	-16.881	1	0	1	-.011	1	-.096	3
369	14	max	70.594	1	1.945	9	-.257	10	0	3	0	10	.143	2
370		min	-5.826	3	-23.552	3	-16.881	1	0	1	-.015	1	-.091	3
371	15	max	70.667	1	1.721	9	-.257	10	0	3	0	10	.147	2
372		min	-5.772	3	-23.755	3	-16.881	1	0	1	-.018	1	-.086	3
373	16	max	67.419	2	13.888	10	-.26	10	0	1	0	10	.15	2
374		min	-33.81	3	-49.203	3	-17.053	1	0	10	-.022	1	-.081	3
375	17	max	67.491	2	13.663	10	-.26	10	0	1	0	10	.148	2
376		min	-33.756	3	-49.405	3	-17.053	1	0	10	-.026	1	-.07	3
377	18	max	-1.867	15	342.464	2	-.26	10	0	3	0	10	.075	2
378		min	-59.771	1	-161.891	3	-17.504	1	0	2	-.03	1	-.035	3
379	19	max	-1.845	15	342.194	2	-.26	10	0	3	0	10	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-59.699	1	-162.093	3	-17.504	1	0	2	-.033	1	0	3
381	M5	1	max	142.752	1	1092.563	3	0	10	0	.006	3	0	3
382		min	.763	3	-879.73	1	-45.512	3	0	3	0	10	0	1
383		2	max	142.825	1	1092.361	3	0	10	0	0	9	.19	1
384		min	.817	3	-880	1	-45.512	3	0	3	-.004	3	-.236	3
385		3	max	173.526	1	6.727	9	4.988	3	0	0	1	.378	1
386		min	-39.759	3	-74.962	3	-.165	1	0	1	-.013	3	-.468	3
387		4	max	173.598	1	6.502	9	4.988	3	0	0	1	.382	1
388		min	-39.704	3	-75.164	3	-.165	1	0	1	-.012	3	-.452	3
389		5	max	173.671	1	6.278	9	4.988	3	0	0	1	.386	1
390		min	-39.65	3	-75.367	3	-.165	1	0	1	-.011	3	-.436	3
391		6	max	173.743	1	6.053	9	4.988	3	0	0	1	.391	1
392		min	-39.596	3	-75.569	3	-.165	1	0	1	-.01	3	-.419	3
393		7	max	173.815	1	5.828	9	4.988	3	0	0	1	.395	1
394		min	-39.542	3	-75.771	3	-.165	1	0	1	-.009	3	-.403	3
395		8	max	173.887	1	5.603	9	4.988	3	0	0	1	.399	1
396		min	-39.488	3	-75.974	3	-.165	1	0	1	-.008	3	-.386	3
397		9	max	173.96	1	5.378	9	4.988	3	0	0	1	.405	2
398		min	-39.433	3	-76.176	3	-.165	1	0	1	-.006	3	-.37	3
399		10	max	174.032	1	5.154	9	4.988	3	0	0	2	.417	2
400		min	-39.379	3	-76.378	3	-.165	1	0	1	-.005	3	-.353	3
401		11	max	174.104	1	4.929	9	4.988	3	0	0	2	.429	2
402		min	-39.325	3	-76.58	3	-.165	1	0	1	-.004	3	-.337	3
403		12	max	174.177	1	4.704	9	4.988	3	0	0	2	.441	2
404		min	-39.271	3	-76.783	3	-.165	1	0	1	-.003	3	-.32	3
405		13	max	174.249	1	4.479	9	4.988	3	0	0	2	.453	2
406		min	-39.217	3	-76.985	3	-.165	1	0	1	-.002	3	-.303	3
407		14	max	174.321	1	4.255	9	4.988	3	0	0	2	.466	2
408		min	-39.162	3	-77.187	3	-.165	1	0	1	-.001	3	-.287	3
409		15	max	174.393	1	4.03	9	4.988	3	0	0	3	.478	2
410		min	-39.108	3	-77.39	3	-.165	1	0	1	0	9	-.27	3
411		16	max	223.679	2	64.026	2	4.963	3	0	0	3	.489	2
412		min	-105.977	3	-135.616	3	-.176	1	0	2	0	9	-.253	3
413		17	max	223.751	2	63.756	2	4.963	3	0	.002	3	.475	2
414		min	-105.923	3	-135.818	3	-.176	1	0	2	0	9	-.223	3
415		18	max	-2.678	12	1107.074	2	4.574	3	0	.003	3	.239	2
416		min	-142.916	1	-518.969	3	-.042	1	0	1	0	9	-.112	3
417		19	max	-2.642	12	1106.804	2	4.574	3	0	.004	3	0	3
418		min	-142.844	1	-519.171	3	-.042	1	0	1	0	1	0	2
419	M9	1	max	59.627	1	336.485	3	47.955	3	0	0	10	0	1
420		min	1.841	15	-271.899	1	.262	10	0	1	-.033	1	0	3
421		2	max	59.699	1	336.283	3	47.955	3	0	0	3	.059	1
422		min	1.862	15	-272.168	1	.262	10	0	1	-.029	1	-.073	3
423		3	max	70.054	1	4.404	9	16.592	1	0	.01	3	.117	1
424		min	-6.529	3	-21.25	3	-2.077	3	0	10	-.025	1	-.145	3
425		4	max	70.126	1	4.179	9	16.592	1	0	.01	3	.118	1
426		min	-6.474	3	-21.452	3	-2.077	3	0	10	-.022	1	-.14	3
427		5	max	70.199	1	3.954	9	16.592	1	0	.009	3	.119	1
428		min	-6.42	3	-21.655	3	-2.077	3	0	10	-.018	1	-.135	3
429		6	max	70.271	1	3.729	9	16.592	1	0	.009	3	.119	1
430		min	-6.366	3	-21.857	3	-2.077	3	0	10	-.014	1	-.131	3
431		7	max	70.343	1	3.505	9	16.592	1	0	.008	3	.12	1
432		min	-6.312	3	-22.059	3	-2.077	3	0	10	-.011	1	-.126	3
433		8	max	70.415	1	3.28	9	16.592	1	0	.008	3	.121	1
434		min	-6.258	3	-22.262	3	-2.077	3	0	10	-.007	1	-.121	3
435		9	max	70.488	1	3.055	9	16.592	1	0	.007	3	.124	2
436		min	-6.203	3	-22.464	3	-2.077	3	0	10	-.004	1	-.116	3





Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	70.56	1	2.83	9	16.592	1	0	1	.007	3	.128	2
438		min	-6.149	3	-22.666	3	-2.077	3	0	10	0	1	-.111	3
439	11	max	70.632	1	2.606	9	16.592	1	0	1	.006	3	.131	2
440		min	-6.095	3	-22.868	3	-2.077	3	0	10	0	10	-.106	3
441	12	max	70.705	1	2.381	9	16.592	1	0	1	.007	1	.135	2
442		min	-6.041	3	-23.071	3	-2.077	3	0	10	0	10	-.101	3
443	13	max	70.777	1	2.156	9	16.592	1	0	1	.011	1	.139	2
444		min	-5.986	3	-23.273	3	-2.077	3	0	10	0	10	-.096	3
445	14	max	70.849	1	1.931	9	16.592	1	0	1	.014	1	.143	2
446		min	-5.932	3	-23.475	3	-2.077	3	0	10	0	10	-.091	3
447	15	max	70.921	1	1.706	9	16.592	1	0	1	.018	1	.146	2
448		min	-5.878	3	-23.678	3	-2.077	3	0	10	0	10	-.086	3
449	16	max	67.49	2	13.665	10	16.783	1	0	10	.022	1	.15	2
450		min	-34.372	3	-49.543	3	-2.085	3	0	1	0	10	-.081	3
451	17	max	67.563	2	13.44	10	16.783	1	0	10	.025	1	.148	2
452		min	-34.318	3	-49.745	3	-2.085	3	0	1	0	10	-.07	3
453	18	max	-1.861	15	342.464	2	17.547	1	0	2	.029	1	.075	2
454		min	-59.649	1	-161.886	3	-1.772	3	0	3	0	10	-.035	3
455	19	max	-1.84	15	342.194	2	17.547	1	0	2	.033	1	0	2
456		min	-59.577	1	-162.088	3	-1.772	3	0	3	0	10	0	3
457	M13	1	max	47.953	3	271.714	1	-1.841	15	0	.033	1	0	1
458		min	.262	10	-336.507	3	-59.624	1	0	3	0	10	0	3
459	2	max	47.953	3	192.738	1	-1.398	15	0	1	.008	3	.144	3
460		min	.262	10	-238.423	3	-45.061	1	0	3	-.001	10	-.116	1
461	3	max	47.953	3	113.761	1	-.956	15	0	1	.006	3	.238	3
462		min	.262	10	-140.338	3	-30.499	1	0	3	-.012	1	-.193	1
463	4	max	47.953	3	34.785	1	-.251	10	0	1	.004	3	.284	3
464		min	.262	10	-42.253	3	-15.937	1	0	3	-.024	1	-.23	1
465	5	max	47.953	3	55.832	3	1.043	2	0	1	.002	3	.281	3
466		min	.262	10	-44.191	1	-2.598	3	0	3	-.028	1	-.228	1
467	6	max	47.953	3	153.917	3	13.188	1	0	1	.001	3	.228	3
468		min	.262	10	-123.168	1	-1.954	3	0	3	-.025	1	-.186	1
469	7	max	47.953	3	252.001	3	27.75	1	0	1	0	3	.127	3
470		min	.262	10	-202.144	1	-1.311	3	0	3	-.015	1	-.104	1
471	8	max	47.953	3	350.086	3	42.312	1	0	1	.003	2	.016	1
472		min	.262	10	-281.12	1	-.667	3	0	3	0	12	-.024	3
473	9	max	47.953	3	448.171	3	56.874	1	0	1	.028	1	.177	1
474		min	.262	10	-360.096	1	-.023	3	0	3	0	3	-.223	3
475	10	max	47.953	3	546.256	3	71.436	1	0	1	.06	1	.377	1
476		min	.262	10	-439.073	1	.58	12	0	3	-.006	3	-.472	3
477	11	max	17.045	1	360.096	1	.511	3	0	3	.027	1	.177	1
478		min	.261	10	-448.171	3	-56.76	1	0	1	-.006	3	-.223	3
479	12	max	17.045	1	281.12	1	1.155	3	0	3	.003	2	.016	1
480		min	.261	10	-350.086	3	-42.198	1	0	1	-.006	3	-.024	3
481	13	max	17.045	1	202.144	1	1.798	3	0	3	0	10	.127	3
482		min	.261	10	-252.001	3	-27.636	1	0	1	-.015	1	-.104	1
483	14	max	17.045	1	123.168	1	2.442	3	0	3	0	15	.228	3
484		min	.261	10	-153.917	3	-13.074	1	0	1	-.025	1	-.186	1
485	15	max	17.045	1	44.191	1	3.086	3	0	3	0	15	.281	3
486		min	.261	10	-55.832	3	-1.043	2	0	1	-.028	1	-.228	1
487	16	max	17.045	1	42.253	3	16.051	1	0	3	0	12	.284	3
488		min	.261	10	-34.785	1	.251	10	0	1	-.023	1	-.23	1
489	17	max	17.045	1	140.338	3	30.613	1	0	3	.001	3	.238	3
490		min	.261	10	-113.761	1	.961	15	0	1	-.012	1	-.193	1
491	18	max	17.045	1	238.423	3	45.175	1	0	3	.007	1	.144	3
492		min	.261	10	-192.738	1	1.404	15	0	1	-.001	10	-.116	1
493	19	max	17.045	1	336.507	3	59.737	1	0	3	.033	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	.261	10	-271.714	1	1.846	15	0	1	0	10	0	3
495	M16	1	max	1.773	3	342.272	2	-1.84	15	0	3	.033	1	0	2
496			min	-17.522	1	-162.101	3	-59.581	1	0	2	0	10	0	3
497		2	max	1.773	3	242.711	2	-1.397	15	0	3	.007	1	.069	3
498			min	-17.522	1	-115.245	3	-45.018	1	0	2	-.001	10	-.146	2
499		3	max	1.773	3	143.151	2	-.955	15	0	3	0	12	.115	3
500			min	-17.522	1	-68.389	3	-30.456	1	0	2	-.012	1	-.243	2
501		4	max	1.773	3	43.591	2	-.237	10	0	3	0	15	.138	3
502			min	-17.522	1	-21.533	3	-15.894	1	0	2	-.024	1	-.289	2
503		5	max	1.773	3	25.322	3	1.067	2	0	3	0	15	.137	3
504			min	-17.522	1	-55.969	2	-1.689	3	0	2	-.028	1	-.286	2
505		6	max	1.773	3	72.178	3	13.231	1	0	3	0	15	.112	3
506			min	-17.522	1	-155.529	2	-1.045	3	0	2	-.025	1	-.233	2
507		7	max	1.773	3	119.034	3	27.793	1	0	3	0	10	.065	3
508			min	-17.522	1	-255.09	2	-.401	3	0	2	-.015	1	-.131	2
509		8	max	1.773	3	165.89	3	42.355	1	0	3	.003	2	.022	2
510			min	-17.522	1	-354.65	2	.243	3	0	2	-.004	3	-.007	3
511		9	max	1.773	3	212.746	3	56.917	1	0	3	.028	1	.224	2
512			min	-17.522	1	-454.21	2	.696	12	0	2	-.003	3	-.101	3
513		10	max	-.26	10	-9.183	15	71.48	1	0	15	.06	1	.476	2
514			min	-17.522	1	-553.77	2	-2.185	3	0	2	-.003	3	-.219	3
515		11	max	-.26	10	454.21	2	-1.088	12	0	2	.028	1	.224	2
516			min	-17.48	1	-212.746	3	-56.796	1	0	3	0	3	-.101	3
517		12	max	-.26	10	354.65	2	-.658	12	0	2	.003	2	.022	2
518			min	-17.48	1	-165.89	3	-42.233	1	0	3	0	3	-.007	3
519		13	max	-.26	10	255.09	2	-.229	12	0	2	0	10	.065	3
520			min	-17.48	1	-119.034	3	-27.671	1	0	3	-.015	1	-.131	2
521		14	max	-.26	10	155.529	2	.39	3	0	2	0	12	.112	3
522			min	-17.48	1	-72.178	3	-13.109	1	0	3	-.025	1	-.233	2
523		15	max	-.26	10	55.969	2	1.599	9	0	2	0	12	.137	3
524			min	-17.48	1	-25.322	3	-1.067	2	0	3	-.028	1	-.286	2
525		16	max	-.26	10	21.533	3	16.016	1	0	2	0	3	.138	3
526			min	-17.48	1	-43.591	2	.237	10	0	3	-.023	1	-.289	2
527		17	max	-.26	10	68.389	3	30.578	1	0	2	.001	3	.115	3
528			min	-17.48	1	-143.151	2	.96	15	0	3	-.012	1	-.243	2
529		18	max	-.26	10	115.245	3	45.14	1	0	2	.007	1	.069	3
530			min	-17.48	1	-242.711	2	1.402	15	0	3	-.001	10	-.146	2
531		19	max	-.26	10	162.101	3	59.702	1	0	2	.033	1	0	2
532			min	-17.48	1	-342.272	2	1.845	15	0	3	0	10	0	3
533	M15	1	max	0	1	.923	3	.104	3	0	1	0	1	0	1
534			min	-57.759	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.821	3	.104	3	0	1	0	1	0	1
536			min	-57.813	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.718	3	.104	3	0	1	0	1	0	1
538			min	-57.867	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.616	3	.104	3	0	1	0	1	0	1
540			min	-57.92	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.513	3	.104	3	0	1	0	1	0	1
542			min	-57.974	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.41	3	.104	3	0	1	0	1	0	1
544			min	-58.028	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.308	3	.104	3	0	1	0	3	0	1
546			min	-58.082	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.205	3	.104	3	0	1	0	3	0	1
548			min	-58.136	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.103	3	.104	3	0	1	0	3	0	1
550			min	-58.19	3	0	1	0	1	0	3	0	1	-.001	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.104	3	0	1	0	3	0	1
552		min	-58.244	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.104	3	0	1	0	3	0	1
554		min	-58.298	3	-.103	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.104	3	0	1	0	3	0	1
556		min	-58.352	3	-.205	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.104	3	0	1	0	3	0	1
558		min	-58.406	3	-.308	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.104	3	0	1	0	3	0	1
560		min	-58.46	3	-.41	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.104	3	0	1	0	3	0	1
562		min	-58.514	3	-.513	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.104	3	0	1	0	3	0	1
564		min	-58.568	3	-.616	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.104	3	0	1	0	3	0	1
566		min	-58.622	3	-.718	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.104	3	0	1	0	3	0	1
568		min	-58.676	3	-.821	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.104	3	0	1	0	3	0	1
570		min	-58.73	3	-.923	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.58	4	.039	1	0	3	0	3	0	1
572		min	-57.668	3	0	2	-.04	3	0	1	0	1	0	1
573	2	max	0	2	1.404	4	.039	1	0	3	0	3	0	2
574		min	-57.614	3	0	2	-.04	3	0	1	0	1	0	4
575	3	max	0	2	1.229	4	.039	1	0	3	0	3	0	2
576		min	-57.56	3	0	2	-.04	3	0	1	0	1	0	4
577	4	max	0	2	1.053	4	.039	1	0	3	0	3	0	2
578		min	-57.506	3	0	2	-.04	3	0	1	0	1	-.001	4
579	5	max	0	2	.878	4	.039	1	0	3	0	3	0	2
580		min	-57.452	3	0	2	-.04	3	0	1	0	1	-.001	4
581	6	max	0	2	.702	4	.039	1	0	3	0	3	0	2
582		min	-57.398	3	0	2	-.04	3	0	1	0	1	-.002	4
583	7	max	0	2	.527	4	.039	1	0	3	0	3	0	2
584		min	-57.344	3	0	2	-.04	3	0	1	0	1	-.002	4
585	8	max	0	2	.351	4	.039	1	0	3	0	3	0	2
586		min	-57.29	3	0	2	-.04	3	0	1	0	1	-.002	4
587	9	max	0	2	.176	4	.039	1	0	3	0	3	0	2
588		min	-57.236	3	0	2	-.04	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.039	1	0	3	0	3	0	2
590		min	-57.182	3	0	1	-.04	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.039	1	0	3	0	3	0	2
592		min	-57.128	3	-.176	4	-.04	3	0	1	0	1	-.002	4
593	12	max	.048	13	0	2	.039	1	0	3	0	3	0	2
594		min	-57.074	3	-.351	4	-.04	3	0	1	0	1	-.002	4
595	13	max	.122	13	0	2	.039	1	0	3	0	1	0	2
596		min	-57.02	3	-.527	4	-.04	3	0	1	0	4	-.002	4
597	14	max	.197	13	0	2	.039	1	0	3	0	1	0	2
598		min	-56.966	3	-.702	4	-.04	3	0	1	0	3	-.002	4
599	15	max	.271	13	0	2	.039	1	0	3	0	1	0	2
600		min	-56.912	3	-.878	4	-.04	3	0	1	0	3	-.001	4
601	16	max	.345	13	0	2	.039	1	0	3	0	1	0	2
602		min	-56.858	3	-1.053	4	-.04	3	0	1	0	3	-.001	4
603	17	max	.419	13	0	2	.039	1	0	3	0	1	0	2
604		min	-56.804	3	-1.229	4	-.04	3	0	1	0	3	0	4
605	18	max	.494	13	0	2	.039	1	0	3	0	1	0	2
606		min	-56.75	3	-1.404	4	-.04	3	0	1	0	3	0	4
607	19	max	.585	4	0	2	.039	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-56.696	3	-1.58	4	-.04	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.005	2	.003	1	-4.372e-6	10	NC	3	NC	1	
2			min	-.003	3	-.004	3	-.001	3	-2.418e-4	1	5663.178	2	NC	1	
3			2	max	.002	1	.005	2	.003	1	-4.19e-6	10	NC	3	NC	1
4				min	-.003	3	-.004	3	-.001	3	-2.319e-4	1	6156.926	2	NC	1
5			3	max	.002	1	.004	2	.002	1	-4.007e-6	10	NC	1	NC	1
6				min	-.002	3	-.004	3	-.001	3	-2.22e-4	1	6740.262	2	NC	1
7			4	max	.002	1	.004	2	.002	1	-3.825e-6	10	NC	1	NC	1
8				min	-.002	3	-.004	3	0	3	-2.122e-4	1	7434.48	2	NC	1
9			5	max	.002	1	.004	2	.002	1	-3.643e-6	10	NC	1	NC	1
10				min	-.002	3	-.004	3	0	3	-2.023e-4	1	8267.875	2	NC	1
11		6	max	.001	1	.003	2	.002	1	-3.46e-6	10	NC	1	NC	1	
12			min	-.002	3	-.004	3	0	3	-1.925e-4	1	9278.675	2	NC	1	
13		7	max	.001	1	.003	2	.002	1	-3.278e-6	10	NC	1	NC	1	
14			min	-.002	3	-.003	3	0	3	-1.826e-4	1	NC	1	NC	1	
15		8	max	.001	1	.002	2	.001	1	-3.096e-6	10	NC	1	NC	1	
16			min	-.002	3	-.003	3	0	3	-1.728e-4	1	NC	1	NC	1	
17		9	max	.001	1	.002	2	.001	1	-2.913e-6	10	NC	1	NC	1	
18			min	-.001	3	-.003	3	0	3	-1.629e-4	1	NC	1	NC	1	
19		10	max	.001	1	.002	2	.001	1	-2.731e-6	10	NC	1	NC	1	
20			min	-.001	3	-.003	3	0	3	-1.531e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	0	1	-2.549e-6	10	NC	1	NC	1	
22			min	-.001	3	-.003	3	0	3	-1.432e-4	1	NC	1	NC	1	
23		12	max	0	1	.001	2	0	1	-2.366e-6	10	NC	1	NC	1	
24			min	-.001	3	-.002	3	0	3	-1.333e-4	1	NC	1	NC	1	
25		13	max	0	1	0	2	0	1	-2.184e-6	10	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-1.235e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	0	1	-2.001e-6	10	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-1.136e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-1.819e-6	10	NC	1	NC	1	
30			min	0	3	-.001	3	0	3	-1.038e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.637e-6	10	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-9.392e-5	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.454e-6	10	NC	1	NC	1	
34			min	0	3	0	3	0	3	-8.407e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.272e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-7.421e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-1.09e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-6.435e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.929e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	4.964e-7	10	NC	1	NC	1	
41			2	max	0	9	0	2	0	10	3.874e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	6.355e-7	10	NC	1	NC	1
43			3	max	0	9	0	2	0	10	4.82e-5	1	NC	1	NC	1
44				min	0	2	-.001	3	0	1	7.746e-7	10	NC	1	NC	1
45			4	max	0	9	0	2	0	12	5.766e-5	1	NC	1	NC	1
46				min	0	2	-.002	3	0	1	9.136e-7	10	NC	1	NC	1
47			5	max	0	9	0	2	0	3	6.712e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	1.053e-6	10	NC	1	NC	1
49			6	max	0	9	0	2	0	3	7.658e-5	1	NC	1	NC	1
50				min	0	2	-.003	3	0	1	1.192e-6	10	NC	1	NC	1
51		7	max	0	9	0	2	0	3	8.603e-5	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.004	3	0	9	1.331e-6	10	NC	1	NC	1
53		8 max	0	9	0	2	0	3	9.549e-5	1	NC	1	NC	1
54		min	0	2	-.004	3	0	9	1.47e-6	10	NC	1	NC	1
55		9 max	0	9	0	2	0	3	1.05e-4	1	NC	1	NC	1
56		min	0	2	-.005	3	0	10	1.609e-6	10	NC	1	NC	1
57		10 max	0	9	.001	2	0	1	1.144e-4	1	NC	1	NC	1
58		min	0	2	-.005	3	0	10	1.748e-6	10	NC	1	NC	1
59		11 max	0	9	.002	2	0	1	1.239e-4	1	NC	1	NC	1
60		min	0	2	-.006	3	0	10	1.887e-6	10	NC	1	NC	1
61		12 max	0	9	.002	2	0	1	1.333e-4	1	NC	1	NC	1
62		min	0	2	-.006	3	0	10	2.026e-6	10	NC	1	NC	1
63		13 max	0	9	.003	2	0	1	1.428e-4	1	NC	1	NC	1
64		min	0	2	-.006	3	0	10	2.165e-6	10	NC	1	NC	1
65		14 max	0	9	.003	2	0	1	1.522e-4	1	NC	1	NC	1
66		min	0	2	-.006	3	0	10	2.304e-6	10	NC	1	NC	1
67		15 max	0	9	.004	2	.001	1	1.617e-4	1	NC	1	NC	1
68		min	0	2	-.006	3	0	10	2.443e-6	10	NC	1	NC	1
69		16 max	0	9	.005	2	.001	1	1.712e-4	1	NC	1	NC	1
70		min	0	2	-.006	3	0	10	2.582e-6	10	9217.88	2	NC	1
71		17 max	0	9	.006	2	.002	1	1.806e-4	1	NC	1	NC	1
72		min	0	2	-.006	3	0	10	2.721e-6	10	7826.044	2	NC	1
73		18 max	0	9	.007	2	.002	1	1.901e-4	1	NC	3	NC	1
74		min	0	2	-.006	3	0	10	2.86e-6	10	6759.369	2	NC	1
75		19 max	0	9	.008	2	.002	1	1.995e-4	1	NC	3	NC	1
76		min	0	2	-.006	3	0	10	2.999e-6	10	5932.172	2	NC	1
77	M4	1 max	.002	1	.006	2	0	10	-2.737e-6	10	NC	1	NC	1
78		min	0	3	-.005	3	-.001	1	-2.149e-4	1	NC	1	NC	1
79		2 max	.002	1	.006	2	0	10	-2.737e-6	10	NC	1	NC	1
80		min	0	3	-.005	3	-.001	1	-2.149e-4	1	NC	1	NC	1
81		3 max	.002	1	.005	2	0	10	-2.737e-6	10	NC	1	NC	1
82		min	0	3	-.004	3	-.001	1	-2.149e-4	1	NC	1	NC	1
83		4 max	.001	1	.005	2	0	10	-2.737e-6	10	NC	1	NC	1
84		min	0	3	-.004	3	-.001	1	-2.149e-4	1	NC	1	NC	1
85		5 max	.001	1	.005	2	0	10	-2.737e-6	10	NC	1	NC	1
86		min	0	3	-.004	3	0	1	-2.149e-4	1	NC	1	NC	1
87		6 max	.001	1	.004	2	0	10	-2.737e-6	10	NC	1	NC	1
88		min	0	3	-.003	3	0	1	-2.149e-4	1	NC	1	NC	1
89		7 max	.001	1	.004	2	0	10	-2.737e-6	10	NC	1	NC	1
90		min	0	3	-.003	3	0	1	-2.149e-4	1	NC	1	NC	1
91		8 max	.001	1	.004	2	0	10	-2.737e-6	10	NC	1	NC	1
92		min	0	3	-.003	3	0	1	-2.149e-4	1	NC	1	NC	1
93		9 max	0	1	.003	2	0	10	-2.737e-6	10	NC	1	NC	1
94		min	0	3	-.003	3	0	1	-2.149e-4	1	NC	1	NC	1
95		10 max	0	1	.003	2	0	10	-2.737e-6	10	NC	1	NC	1
96		min	0	3	-.002	3	0	1	-2.149e-4	1	NC	1	NC	1
97		11 max	0	1	.003	2	0	10	-2.737e-6	10	NC	1	NC	1
98		min	0	3	-.002	3	0	1	-2.149e-4	1	NC	1	NC	1
99		12 max	0	1	.002	2	0	10	-2.737e-6	10	NC	1	NC	1
100		min	0	3	-.002	3	0	1	-2.149e-4	1	NC	1	NC	1
101		13 max	0	1	.002	2	0	10	-2.737e-6	10	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-2.149e-4	1	NC	1	NC	1
103		14 max	0	1	.002	2	0	10	-2.737e-6	10	NC	1	NC	1
104		min	0	3	-.001	3	0	1	-2.149e-4	1	NC	1	NC	1
105		15 max	0	1	.001	2	0	10	-2.737e-6	10	NC	1	NC	1
106		min	0	3	-.001	3	0	1	-2.149e-4	1	NC	1	NC	1
107		16 max	0	1	.001	2	0	10	-2.737e-6	10	NC	1	NC	1
108		min	0	3	0	3	0	1	-2.149e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	10	-2.737e-6	10	NC	1	NC	1
110			min	0	3	0	3	0	1	-2.149e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	-2.737e-6	10	NC	1	NC	1
112			min	0	3	0	3	0	1	-2.149e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.737e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-2.149e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.018	2	0	1	2.603e-4	3	NC	3	NC	1
116			min	-.008	3	-.013	3	-.004	3	-8.515e-8	2	1686.906	2	8324.059	3
117		2	max	.006	1	.017	2	0	1	2.546e-4	3	NC	3	NC	1
118			min	-.008	3	-.013	3	-.003	3	-8.064e-8	2	1801.101	2	8918.42	3
119		3	max	.006	1	.016	2	0	1	2.488e-4	3	NC	3	NC	1
120			min	-.008	3	-.012	3	-.003	3	-7.613e-8	2	1931.47	2	9614.413	3
121		4	max	.005	1	.014	2	0	1	2.431e-4	3	NC	3	NC	1
122			min	-.007	3	-.011	3	-.003	3	-6.992e-7	11	2081.257	2	NC	1
123		5	max	.005	1	.013	2	0	1	2.373e-4	3	NC	3	NC	1
124			min	-.007	3	-.011	3	-.003	3	-1.743e-6	11	2254.642	2	NC	1
125		6	max	.005	1	.012	2	0	1	2.315e-4	3	NC	3	NC	1
126			min	-.006	3	-.01	3	-.002	3	-3.032e-6	1	2457.096	2	NC	1
127		7	max	.004	1	.011	2	0	1	2.258e-4	3	NC	3	NC	1
128			min	-.006	3	-.009	3	-.002	3	-5.382e-6	1	2695.92	2	NC	1
129		8	max	.004	1	.01	2	0	1	2.2e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.002	3	-7.733e-6	1	2981.079	2	NC	1
131		9	max	.004	1	.009	2	0	1	2.143e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	-.002	3	-1.008e-5	1	3326.531	2	NC	1
133		10	max	.003	1	.008	2	0	1	2.085e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	-.001	3	-1.243e-5	1	3752.444	2	NC	1
135		11	max	.003	1	.007	2	0	1	2.027e-4	3	NC	3	NC	1
136			min	-.004	3	-.007	3	-.001	3	-1.478e-5	1	4289.076	2	NC	1
137		12	max	.002	1	.006	2	0	1	1.97e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	-.001	3	-1.713e-5	1	4983.983	2	NC	1
139		13	max	.002	1	.005	2	0	1	1.912e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-1.948e-5	1	5916.431	2	NC	1
141		14	max	.002	1	.004	2	0	1	1.855e-4	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-2.183e-5	1	7229.102	2	NC	1
143		15	max	.001	1	.003	2	0	1	1.797e-4	3	NC	1	NC	1
144			min	-.002	3	-.003	3	0	3	-2.418e-5	1	9207.366	2	NC	1
145		16	max	.001	1	.002	2	0	1	1.739e-4	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-2.654e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.682e-4	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-2.889e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.624e-4	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-3.124e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.567e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.359e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.521e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-7.095e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	1.39e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	1	-5.532e-5	3	NC	1	NC	1
157		3	max	0	9	.002	2	0	3	1.259e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-3.969e-5	3	NC	1	NC	1
159		4	max	0	9	.003	2	0	3	1.128e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-2.406e-5	3	NC	1	NC	1
161		5	max	0	9	.004	2	.001	3	9.975e-6	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-8.429e-6	3	NC	1	NC	1
163		6	max	0	9	.005	2	.002	3	8.666e-6	1	NC	1	NC	1
164			min	0	2	-.007	3	0	1	0	2	8553.554	2	NC	1
165		7	max	0	9	.007	2	.002	3	2.283e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166		min	0	2	-.008	3	0	1	0	2	7075.993	2	NC	1
167	8	max	0	9	.008	2	.002	3	3.846e-5	3	NC	3	NC	1
168		min	0	2	-.01	3	0	1	-3.49e-8	13	5988.877	2	NC	1
169	9	max	0	9	.009	2	.002	3	5.409e-5	3	NC	3	NC	1
170		min	0	2	-.011	3	0	1	-3.23e-7	9	5151.328	2	NC	1
171	10	max	0	9	.01	2	.002	3	6.972e-5	3	NC	3	NC	1
172		min	-.001	2	-.012	3	0	1	-1.579e-6	9	4485.159	2	NC	1
173	11	max	0	9	.012	2	.002	3	8.535e-5	3	NC	3	NC	1
174		min	-.001	2	-.013	3	0	1	-2.836e-6	9	3943.235	2	NC	1
175	12	max	0	9	.013	2	.002	3	1.01e-4	3	NC	3	NC	1
176		min	-.001	2	-.014	3	0	1	-4.092e-6	9	3495.187	2	NC	1
177	13	max	0	9	.015	2	.002	3	1.166e-4	3	NC	3	NC	1
178		min	-.001	2	-.015	3	0	1	-5.349e-6	9	3120.341	2	NC	1
179	14	max	0	9	.016	2	.002	3	1.322e-4	3	NC	3	NC	1
180		min	-.002	2	-.016	3	0	1	-6.605e-6	9	2803.965	2	NC	1
181	15	max	0	9	.018	2	.002	3	1.479e-4	3	NC	3	NC	1
182		min	-.002	2	-.017	3	0	1	-7.862e-6	9	2535.166	2	NC	1
183	16	max	0	9	.02	2	.002	3	1.635e-4	3	NC	3	NC	1
184		min	-.002	2	-.018	3	0	1	-9.118e-6	9	2305.65	2	NC	1
185	17	max	.001	9	.022	2	.002	3	1.791e-4	3	NC	3	NC	1
186		min	-.002	2	-.018	3	0	1	-1.037e-5	9	2108.956	2	NC	1
187	18	max	.001	9	.024	2	.002	3	1.948e-4	3	NC	3	NC	1
188		min	-.002	2	-.019	3	0	1	-1.163e-5	9	1939.966	2	NC	1
189	19	max	.001	9	.026	2	.002	3	2.104e-4	3	NC	3	NC	1
190		min	-.002	2	-.02	3	0	1	-1.289e-5	9	1794.578	2	NC	1
191	M8	1	max	.005	1	.02	2	0	-7.237e-8	10	NC	1	NC	1
192		min	-.002	3	-.015	3	-.001	3	-1.674e-4	3	NC	1	NC	1
193	2	max	.005	1	.019	2	0	1	-7.237e-8	10	NC	1	NC	1
194		min	-.002	3	-.014	3	-.001	3	-1.674e-4	3	NC	1	NC	1
195	3	max	.005	1	.018	2	0	1	-7.237e-8	10	NC	1	NC	1
196		min	-.002	3	-.013	3	-.001	3	-1.674e-4	3	NC	1	NC	1
197	4	max	.004	1	.017	2	0	1	-7.237e-8	10	NC	1	NC	1
198		min	-.002	3	-.012	3	0	3	-1.674e-4	3	NC	1	NC	1
199	5	max	.004	1	.016	2	0	1	-7.237e-8	10	NC	1	NC	1
200		min	-.001	3	-.012	3	0	3	-1.674e-4	3	NC	1	NC	1
201	6	max	.004	1	.015	2	0	1	-7.237e-8	10	NC	1	NC	1
202		min	-.001	3	-.011	3	0	3	-1.674e-4	3	NC	1	NC	1
203	7	max	.004	1	.014	2	0	1	-7.237e-8	10	NC	1	NC	1
204		min	-.001	3	-.01	3	0	3	-1.674e-4	3	NC	1	NC	1
205	8	max	.003	1	.012	2	0	1	-7.237e-8	10	NC	1	NC	1
206		min	-.001	3	-.009	3	0	3	-1.674e-4	3	NC	1	NC	1
207	9	max	.003	1	.011	2	0	1	-7.237e-8	10	NC	1	NC	1
208		min	-.001	3	-.008	3	0	3	-1.674e-4	3	NC	1	NC	1
209	10	max	.003	1	.01	2	0	1	-7.237e-8	10	NC	1	NC	1
210		min	0	3	-.007	3	0	3	-1.674e-4	3	NC	1	NC	1
211	11	max	.002	1	.009	2	0	1	-7.237e-8	10	NC	1	NC	1
212		min	0	3	-.007	3	0	3	-1.674e-4	3	NC	1	NC	1
213	12	max	.002	1	.008	2	0	1	-7.237e-8	10	NC	1	NC	1
214		min	0	3	-.006	3	0	3	-1.674e-4	3	NC	1	NC	1
215	13	max	.002	1	.007	2	0	1	-7.237e-8	10	NC	1	NC	1
216		min	0	3	-.005	3	0	3	-1.674e-4	3	NC	1	NC	1
217	14	max	.001	1	.006	2	0	1	-7.237e-8	10	NC	1	NC	1
218		min	0	3	-.004	3	0	3	-1.674e-4	3	NC	1	NC	1
219	15	max	.001	1	.005	2	0	1	-7.237e-8	10	NC	1	NC	1
220		min	0	3	-.003	3	0	3	-1.674e-4	3	NC	1	NC	1
221	16	max	0	1	.003	2	0	1	-7.237e-8	10	NC	1	NC	1
222		min	0	3	-.002	3	0	3	-1.674e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.002	2	0	1	-7.237e-8	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.674e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-7.237e-8	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.674e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-7.237e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.674e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.005	2	0	3	2.575e-4	1	NC	3	NC	1
230			min	-.002	3	-.004	3	-.001	1	-3.492e-4	3	5674.732	2	NC	1
231		2	max	.002	1	.005	2	0	3	2.45e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.001	1	-3.393e-4	3	6169.746	2	NC	1
233		3	max	.002	1	.004	2	0	3	2.325e-4	1	NC	1	NC	1
234			min	-.002	3	-.004	3	0	1	-3.295e-4	3	6754.625	2	NC	1
235		4	max	.002	1	.004	2	0	3	2.2e-4	1	NC	1	NC	1
236			min	-.002	3	-.004	3	0	1	-3.197e-4	3	7450.741	2	NC	1
237		5	max	.002	1	.004	2	0	3	2.075e-4	1	NC	1	NC	1
238			min	-.002	3	-.004	3	0	1	-3.098e-4	3	8286.493	2	NC	1
239		6	max	.001	1	.003	2	0	3	1.95e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	0	1	-3.e-4	3	9300.257	2	NC	1
241		7	max	.001	1	.003	2	0	3	1.825e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	0	1	-2.901e-4	3	NC	1	NC	1
243		8	max	.001	1	.002	2	0	3	1.7e-4	1	NC	1	NC	1
244			min	-.001	3	-.003	3	0	1	-2.803e-4	3	NC	1	NC	1
245		9	max	.001	1	.002	2	0	3	1.575e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-2.705e-4	3	NC	1	NC	1
247		10	max	.001	1	.002	2	0	3	1.45e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-2.606e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	1.326e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-2.508e-4	3	NC	1	NC	1
251		12	max	0	1	.001	2	0	3	1.201e-4	1	NC	1	NC	1
252			min	0	3	-.002	3	0	1	-2.41e-4	3	NC	1	NC	1
253		13	max	0	1	0	2	0	3	1.076e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-2.311e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	9.506e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-2.213e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	8.257e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.115e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	7.007e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-2.016e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	5.758e-5	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-1.918e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	4.508e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.82e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.258e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.721e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.856e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.506e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	1	6.3e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.749e-5	1	NC	1	NC	1
271		3	max	0	9	0	2	0	1	4.744e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-3.992e-5	1	NC	1	NC	1
273		4	max	0	9	0	2	0	2	3.188e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.235e-5	1	NC	1	NC	1
275		5	max	0	9	0	2	0	2	1.632e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-6.478e-5	1	NC	1	NC	1
277		6	max	0	9	0	2	0	2	7.595e-7	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-7.72e-5	1	NC	1	NC	1
279		7	max	0	9	0	2	0	10	-1.286e-6	10	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	2.127e-4	3	NC	1	NC	1
338			min	0	3	0	3	0	10	2.593e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.127e-4	3	NC	1	NC	1
340			min	0	3	0	3	0	10	2.593e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.127e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	2.593e-6	10	NC	1	NC	1
343	M1	1	max	.004	3	.021	3	.002	3	8.312e-3	1	NC	1	NC	1
344			min	-.005	2	-.019	1	0	1	-1.009e-2	3	NC	1	NC	1
345		2	max	.004	3	.011	3	.002	3	4.053e-3	1	NC	4	NC	1
346			min	-.005	2	-.01	1	-.002	1	-4.968e-3	3	5122.109	3	NC	1
347		3	max	.004	3	.002	3	.001	3	6.331e-5	3	NC	4	NC	1
348			min	-.005	2	-.002	1	-.003	1	-1.265e-4	1	2659.86	3	NC	1
349		4	max	.004	3	.006	2	0	3	6.152e-5	3	NC	4	NC	1
350			min	-.005	2	-.005	3	-.003	1	-1.022e-4	1	1884.142	1	NC	1
351		5	max	.004	3	.012	2	0	3	5.973e-5	3	NC	4	NC	1
352			min	-.005	2	-.011	3	-.003	1	-7.791e-5	1	1503.421	2	NC	1
353		6	max	.004	3	.017	2	0	3	5.794e-5	3	NC	5	NC	1
354			min	-.005	2	-.016	3	-.003	1	-5.361e-5	1	1281.902	2	NC	1
355		7	max	.004	3	.021	2	0	3	5.615e-5	3	NC	5	NC	1
356			min	-.005	2	-.019	3	-.003	1	-2.93e-5	1	1145.881	2	NC	1
357		8	max	.004	3	.024	2	0	3	5.437e-5	3	NC	5	NC	1
358			min	-.005	2	-.022	3	-.002	1	-1.195e-5	9	1061.25	2	NC	1
359		9	max	.004	3	.026	2	0	3	5.258e-5	3	NC	5	NC	1
360			min	-.005	2	-.023	3	-.002	1	4.216e-7	15	1011.944	2	NC	1
361		10	max	.004	3	.027	2	0	3	5.079e-5	3	NC	5	NC	1
362			min	-.005	2	-.023	3	0	1	1.22e-6	15	990.355	2	NC	1
363		11	max	.004	3	.026	2	0	3	6.791e-5	1	NC	5	NC	1
364			min	-.005	2	-.022	3	0	9	1.583e-6	10	993.826	2	NC	1
365		12	max	.004	3	.025	2	0	1	9.221e-5	1	NC	5	NC	1
366			min	-.005	2	-.02	3	0	10	1.864e-6	10	1023.64	2	NC	1
367		13	max	.004	3	.022	2	.001	1	1.165e-4	1	NC	5	NC	1
368			min	-.005	2	-.018	3	0	10	2.146e-6	10	1085.661	2	NC	1
369		14	max	.004	3	.018	2	.002	1	1.408e-4	1	NC	5	NC	1
370			min	-.005	2	-.014	3	0	10	2.427e-6	10	1193.239	2	NC	1
371		15	max	.004	3	.012	2	.002	1	1.651e-4	1	NC	4	NC	1
372			min	-.005	2	-.01	3	0	10	2.709e-6	10	1375.456	2	NC	1
373		16	max	.005	3	.005	2	.002	1	1.833e-4	1	NC	4	NC	1
374			min	-.005	2	-.004	3	0	10	2.939e-6	10	1702.666	2	NC	1
375		17	max	.005	3	.002	3	.001	1	5.656e-5	1	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	1.643e-6	15	2393.406	2	NC	1
377		18	max	.005	3	.008	3	0	1	5.173e-3	2	NC	4	NC	1
378			min	-.005	2	-.013	2	0	10	-2.531e-3	3	4623.872	2	NC	1
379		19	max	.005	3	.015	3	0	3	1.042e-2	2	NC	1	NC	1
380			min	-.005	2	-.023	2	0	1	-5.146e-3	3	NC	1	NC	1
381	M5	1	max	.014	3	.066	3	.002	3	2.363e-6	3	NC	1	NC	1
382			min	-.018	2	-.062	1	0	1	0	1	NC	1	NC	1
383		2	max	.014	3	.036	3	.003	3	6.847e-5	3	NC	4	NC	1
384			min	-.018	2	-.033	1	0	1	-1.977e-5	1	1577.388	1	NC	1
385		3	max	.014	3	.008	3	.004	3	1.333e-4	3	NC	5	NC	1
386			min	-.018	2	-.005	1	0	1	-3.916e-5	1	812.841	1	NC	1
387		4	max	.014	3	.019	2	.004	3	1.316e-4	3	NC	5	NC	1
388			min	-.018	2	-.016	3	0	1	-3.646e-5	1	573.227	1	NC	1
389		5	max	.014	3	.039	2	.005	3	1.299e-4	3	NC	5	NC	1
390			min	-.018	2	-.035	3	0	1	-3.425e-5	9	457.905	1	NC	1
391		6	max	.014	3	.055	2	.005	3	1.281e-4	3	NC	5	NC	1
392			min	-.018	2	-.05	3	0	1	-3.207e-5	9	392.563	1	NC	1
393		7	max	.014	3	.069	2	.005	3	1.264e-4	3	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394		min	-.018	2	-.061	3	0	1	-2.988e-5	9	352.324	2	NC	1
395	8	max	.014	3	.078	2	.005	3	1.247e-4	3	NC	5	NC	1
396		min	-.018	2	-.069	3	0	1	-2.77e-5	9	326.181	2	NC	1
397	9	max	.014	3	.085	2	.005	3	1.23e-4	3	NC	5	NC	1
398		min	-.018	2	-.073	3	0	1	-2.551e-5	9	310.93	2	NC	1
399	10	max	.014	3	.087	2	.004	3	1.213e-4	3	NC	5	NC	1
400		min	-.018	2	-.073	3	0	1	-2.333e-5	9	304.221	2	NC	1
401	11	max	.014	3	.086	2	.004	3	1.195e-4	3	NC	5	NC	1
402		min	-.018	2	-.07	3	0	1	-2.114e-5	9	305.232	2	NC	1
403	12	max	.014	3	.08	2	.004	3	1.178e-4	3	NC	5	NC	1
404		min	-.018	2	-.065	3	0	1	-1.896e-5	9	314.352	2	NC	1
405	13	max	.014	3	.071	2	.003	3	1.161e-4	3	NC	5	NC	1
406		min	-.018	2	-.056	3	0	1	-1.677e-5	9	333.382	2	NC	1
407	14	max	.014	3	.057	2	.003	3	1.144e-4	3	NC	5	NC	1
408		min	-.018	2	-.044	3	0	1	-1.459e-5	9	366.429	2	NC	1
409	15	max	.014	3	.039	2	.002	3	1.126e-4	3	NC	5	NC	1
410		min	-.018	2	-.03	3	0	1	-1.24e-5	9	422.442	2	NC	1
411	16	max	.014	3	.017	2	.002	3	1.085e-4	3	NC	5	NC	1
412		min	-.018	2	-.014	3	0	1	-1.124e-5	9	523.089	2	NC	1
413	17	max	.014	3	.005	3	.001	3	4.78e-5	3	NC	5	NC	1
414		min	-.018	2	-.01	2	0	1	-3.846e-5	1	735.861	2	NC	1
415	18	max	.014	3	.026	3	0	3	2.339e-5	3	NC	4	NC	1
416		min	-.018	2	-.041	2	0	1	-1.976e-5	1	1422.24	2	NC	1
417	19	max	.014	3	.049	3	0	3	0	15	NC	1	NC	1
418		min	-.018	2	-.075	2	0	1	-3.32e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.02	.002	3	1.01e-2	3	NC	1	NC	1
420		min	-.005	2	-.019	1	-.001	1	-8.312e-3	1	NC	1	NC	1
421	2	max	.005	3	.011	3	0	3	5.017e-3	3	NC	4	NC	1
422		min	-.005	2	-.01	1	0	9	-4.098e-3	1	5124.207	3	NC	1
423	3	max	.004	3	.002	3	.001	1	3.737e-5	1	NC	4	NC	1
424		min	-.005	2	-.002	1	0	3	8.374e-7	10	2660.98	3	NC	1
425	4	max	.005	3	.006	2	.002	1	1.974e-5	3	NC	4	NC	1
426		min	-.005	2	-.005	3	-.001	3	3.798e-7	15	1885.39	1	NC	1
427	5	max	.005	3	.012	2	.002	1	1.079e-5	2	NC	4	NC	1
428		min	-.005	2	-.011	3	-.002	3	-1.093e-5	9	1503.718	2	NC	1
429	6	max	.005	3	.017	2	.002	1	4.546e-6	2	NC	5	NC	1
430		min	-.005	2	-.016	3	-.002	3	-2.44e-5	9	1282.168	2	NC	1
431	7	max	.005	3	.021	2	.001	1	-3.2e-7	10	NC	5	NC	1
432		min	-.005	2	-.019	3	-.003	3	-4.349e-5	1	1146.129	2	NC	1
433	8	max	.005	3	.024	2	0	1	-6.093e-7	10	NC	5	NC	1
434		min	-.005	2	-.022	3	-.003	3	-6.371e-5	1	1061.49	2	NC	1
435	9	max	.005	3	.026	2	0	2	-8.987e-7	10	NC	5	NC	1
436		min	-.005	2	-.023	3	-.003	3	-8.392e-5	1	1012.182	2	NC	1
437	10	max	.005	3	.027	2	0	2	-1.188e-6	10	NC	5	NC	1
438		min	-.005	2	-.023	3	-.003	3	-1.041e-4	1	990.596	2	NC	1
439	11	max	.005	3	.026	2	0	10	-1.477e-6	10	NC	5	NC	1
440		min	-.005	2	-.022	3	-.003	3	-1.244e-4	1	994.075	2	NC	1
441	12	max	.005	3	.025	2	0	10	-1.767e-6	10	NC	5	NC	1
442		min	-.005	2	-.02	3	-.003	3	-1.446e-4	1	1023.904	2	NC	1
443	13	max	.005	3	.022	2	0	10	-2.056e-6	10	NC	5	NC	1
444		min	-.005	2	-.018	3	-.003	3	-1.648e-4	1	1085.947	2	NC	1
445	14	max	.005	3	.018	2	0	10	-2.345e-6	10	NC	5	NC	1
446		min	-.005	2	-.014	3	-.003	1	-1.85e-4	1	1193.56	2	NC	1
447	15	max	.005	3	.012	2	0	10	-2.635e-6	10	NC	4	NC	1
448		min	-.005	2	-.01	3	-.003	1	-2.052e-4	1	1375.828	2	NC	1
449	16	max	.005	3	.005	2	0	10	-2.879e-6	10	NC	4	NC	1
450		min	-.005	2	-.004	3	-.003	1	-2.211e-4	1	1703.122	2	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	10	-2.056e-6	10	NC	4	NC	1
452			min	-.005	2	-.003	2	-.003	1	-1.348e-4	1	2393.997	2	NC	1
453		18	max	.005	3	.008	3	0	10	2.54e-3	3	NC	4	NC	1
454			min	-.005	2	-.013	2	-.002	1	-5.174e-3	2	4624.976	2	NC	1
455		19	max	.005	3	.015	3	0	3	5.145e-3	3	NC	1	NC	1
456			min	-.005	2	-.023	2	0	1	-1.042e-2	2	NC	1	NC	1
457	M13	1	max	.001	1	.02	3	.005	3	3.561e-3	3	NC	1	NC	1
458			min	-.002	3	-.019	1	-.005	2	-3.382e-3	1	NC	1	NC	1
459		2	max	.001	1	.079	3	.003	9	4.435e-3	3	NC	4	NC	1
460			min	-.002	3	-.067	1	-.004	2	-4.238e-3	1	1844.397	3	NC	1
461		3	max	.001	1	.128	3	.008	1	5.31e-3	3	NC	5	NC	2
462			min	-.002	3	-.108	1	-.003	10	-5.093e-3	1	1007.242	3	9085.601	1
463		4	max	0	1	.16	3	.013	1	6.184e-3	3	NC	5	NC	2
464			min	-.002	3	-.134	1	-.004	10	-5.949e-3	1	775.318	3	6459.384	1
465		5	max	0	1	.172	3	.015	1	7.058e-3	3	NC	5	NC	2
466			min	-.002	3	-.145	1	-.004	10	-6.804e-3	1	712.655	3	5946.37	1
467		6	max	0	1	.165	3	.012	1	7.933e-3	3	NC	5	NC	2
468			min	-.002	3	-.14	1	-.005	10	-7.66e-3	1	748.914	3	6934.703	1
469		7	max	0	1	.141	3	.008	9	8.807e-3	3	NC	5	NC	1
470			min	-.002	3	-.122	1	-.008	2	-8.516e-3	1	892.567	3	NC	1
471		8	max	0	1	.109	3	.01	3	9.682e-3	3	NC	5	NC	1
472			min	-.002	3	-.097	1	-.013	2	-9.371e-3	1	1212.069	3	NC	1
473		9	max	0	1	.08	3	.012	3	1.056e-2	3	NC	4	NC	1
474			min	-.002	3	-.073	1	-.016	2	-1.023e-2	1	1824.893	3	9819.622	2
475		10	max	0	1	.066	3	.014	3	1.143e-2	3	NC	4	NC	1
476			min	-.002	3	-.062	1	-.018	2	-1.108e-2	1	2380.706	3	8548.057	2
477		11	max	0	1	.08	3	.015	3	1.056e-2	3	NC	4	NC	1
478			min	-.002	3	-.073	1	-.016	2	-1.023e-2	1	1824.892	3	9819.681	2
479		12	max	0	1	.11	3	.015	3	9.683e-3	3	NC	5	NC	1
480			min	-.002	3	-.097	1	-.013	2	-9.372e-3	1	1212.069	3	9844.212	3
481		13	max	0	1	.141	3	.015	3	8.81e-3	3	NC	5	NC	1
482			min	-.002	3	-.122	1	-.008	2	-8.516e-3	1	892.567	3	NC	1
483		14	max	0	1	.165	3	.014	3	7.936e-3	3	NC	5	NC	2
484			min	-.002	3	-.14	1	-.005	10	-7.661e-3	1	748.914	3	6927.21	1
485		15	max	0	1	.172	3	.015	1	7.063e-3	3	NC	5	NC	2
486			min	-.002	3	-.145	1	-.004	10	-6.806e-3	1	712.654	3	5950.048	1
487		16	max	0	1	.16	3	.013	1	6.189e-3	3	NC	5	NC	2
488			min	-.002	3	-.134	1	-.004	10	-5.95e-3	1	775.317	3	6473.076	1
489		17	max	0	1	.128	3	.008	1	5.315e-3	3	NC	5	NC	2
490			min	-.002	3	-.108	1	-.003	10	-5.095e-3	1	1007.242	3	9121.788	1
491		18	max	0	1	.079	3	.006	3	4.442e-3	3	NC	4	NC	1
492			min	-.002	3	-.067	1	-.004	2	-4.239e-3	1	1844.397	3	NC	1
493		19	max	0	1	.021	3	.004	3	3.568e-3	3	NC	1	NC	1
494			min	-.002	3	-.019	1	-.005	2	-3.384e-3	1	NC	1	NC	1
495	M16	1	max	0	1	.015	3	.005	3	3.94e-3	2	NC	1	NC	1
496			min	0	3	-.023	2	-.005	2	-2.676e-3	3	NC	1	NC	1
497		2	max	0	1	.045	3	.006	3	4.926e-3	2	NC	4	NC	1
498			min	0	3	-.084	2	-.004	2	-3.318e-3	3	1786.87	2	NC	1
499		3	max	0	1	.07	3	.008	3	5.913e-3	2	NC	5	NC	2
500			min	0	3	-.134	2	-.003	10	-3.959e-3	3	974.552	2	9165.977	1
501		4	max	0	1	.088	3	.013	1	6.899e-3	2	NC	5	NC	2
502			min	0	3	-.167	2	-.004	10	-4.601e-3	3	748.406	2	6523.256	1
503		5	max	0	1	.095	3	.014	1	7.886e-3	2	NC	5	NC	2
504			min	0	3	-.181	2	-.004	10	-5.243e-3	3	685.303	2	6020.568	1
505		6	max	0	1	.093	3	.013	3	8.872e-3	2	NC	5	NC	2
506			min	0	3	-.174	2	-.006	10	-5.884e-3	3	715.694	2	7062.316	1
507		7	max	0	1	.083	3	.014	3	9.858e-3	2	NC	5	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.151	2	-.009	2	-6.526e-3	3	843.913	2	NC	1
509	8	max	0	1	.069	3	.015	3	1.084e-2	2	NC	5	NC	1
510		min	0	3	-.119	2	-.013	2	-7.167e-3	3	1124.039	2	NC	1
511	9	max	0	1	.055	3	.014	3	1.183e-2	2	NC	4	NC	1
512		min	0	3	-.089	2	-.017	2	-7.809e-3	3	1635.925	2	9606.903	2
513	10	max	0	1	.049	3	.014	3	1.282e-2	2	NC	4	NC	1
514		min	0	3	-.075	2	-.018	2	-8.451e-3	3	2072.857	2	8382.804	2
515	11	max	0	1	.055	3	.013	3	1.183e-2	2	NC	4	NC	1
516		min	0	3	-.089	2	-.017	2	-7.808e-3	3	1635.925	2	9606.944	2
517	12	max	0	1	.069	3	.012	3	1.085e-2	2	NC	5	NC	1
518		min	0	3	-.119	2	-.013	2	-7.166e-3	3	1124.039	2	NC	1
519	13	max	0	1	.083	3	.011	3	9.859e-3	2	NC	5	NC	1
520		min	0	3	-.151	2	-.009	2	-6.524e-3	3	843.913	2	NC	1
521	14	max	0	1	.093	3	.012	1	8.873e-3	2	NC	5	NC	2
522		min	0	3	-.174	2	-.006	10	-5.881e-3	3	715.694	2	7071.903	1
523	15	max	0	1	.095	3	.014	1	7.886e-3	2	NC	5	NC	2
524		min	0	3	-.181	2	-.004	10	-5.239e-3	3	685.303	2	6037.655	1
525	16	max	0	1	.088	3	.013	1	6.9e-3	2	NC	5	NC	2
526		min	0	3	-.167	2	-.004	10	-4.597e-3	3	748.406	2	6552.035	1
527	17	max	0	1	.07	3	.008	1	5.914e-3	2	NC	5	NC	2
528		min	0	3	-.134	2	-.003	10	-3.954e-3	3	974.552	2	9226.731	1
529	18	max	0	1	.045	3	.005	3	4.928e-3	2	NC	4	NC	1
530		min	0	3	-.084	2	-.004	2	-3.312e-3	3	1786.87	2	NC	1
531	19	max	0	1	.015	3	.005	3	3.941e-3	2	NC	1	NC	1
532		min	0	3	-.023	2	-.005	2	-2.67e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.082e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.587e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.592e-4	3	NC	1	NC	1
536		min	0	2	-.003	4	0	3	-5.035e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	1	1.21e-3	3	NC	1	NC	1
538		min	0	2	-.006	4	-.003	3	-9.612e-4	2	NC	1	NC	1
539	4	max	0	3	-.002	15	.006	1	1.661e-3	3	NC	3	NC	4
540		min	0	2	-.009	4	-.006	3	-1.431e-3	1	7138.056	4	7227.603	3
541	5	max	0	3	-.003	15	.009	1	2.112e-3	3	NC	5	NC	4
542		min	0	2	-.011	4	-.01	3	-1.905e-3	1	5569.897	4	4702.473	3
543	6	max	0	3	-.003	15	.013	1	2.563e-3	3	NC	5	NC	4
544		min	0	2	-.013	4	-.014	3	-2.378e-3	1	4687.657	4	3403.558	3
545	7	max	0	3	-.004	15	.017	1	3.014e-3	3	NC	5	NC	4
546		min	0	2	-.015	4	-.018	3	-2.852e-3	1	4157.106	4	2649.076	3
547	8	max	0	3	-.004	15	.021	1	3.465e-3	3	NC	5	NC	4
548		min	-.001	2	-.016	4	-.022	3	-3.325e-3	1	3838.696	4	2176.954	3
549	9	max	0	3	-.004	15	.025	1	3.916e-3	3	NC	5	NC	4
550		min	-.001	2	-.017	4	-.026	3	-3.798e-3	1	3667.309	4	1868.896	3
551	10	max	0	3	-.004	15	.028	1	4.367e-3	3	NC	15	NC	4
552		min	-.001	2	-.017	4	-.029	3	-4.272e-3	1	3613.09	4	1665.813	3
553	11	max	0	3	-.004	15	.03	1	4.818e-3	3	NC	5	NC	5
554		min	-.001	2	-.017	4	-.032	3	-4.745e-3	1	3667.309	4	1536.766	3
555	12	max	0	3	-.004	15	.031	1	5.269e-3	3	NC	5	NC	5
556		min	-.002	2	-.017	4	-.033	3	-5.219e-3	1	3838.696	4	1466.344	3
557	13	max	0	3	-.004	15	.03	1	5.72e-3	3	NC	5	NC	5
558		min	-.002	2	-.015	4	-.032	3	-5.692e-3	1	4157.106	4	1449.857	3
559	14	max	0	3	-.003	15	.028	1	6.171e-3	3	NC	5	NC	5
560		min	-.002	2	-.014	4	-.03	3	-6.165e-3	1	4687.657	4	1493.326	3
561	15	max	0	3	-.002	12	.025	1	6.622e-3	3	NC	5	NC	4
562		min	-.002	2	-.012	4	-.026	3	-6.639e-3	1	5569.897	4	1619.694	3
563	16	max	0	3	-.001	12	.019	1	7.073e-3	3	NC	3	NC	4
564		min	-.002	2	-.009	4	-.02	3	-7.112e-3	1	7138.056	4	1891.611	3

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	0	3	.01	1	7.524e-3	3	NC	1	NC	4
566			min	-0.002	2	-0.007	4	-0.011	3	-7.586e-3	1	NC	1	2505.907	3
567		18	max	0	3	.002	3	.001	9	7.975e-3	3	NC	1	NC	4
568			min	-.002	2	-.004	4	-.005	2	-8.059e-3	1	NC	1	4458.592	3
569		19	max	0	3	.004	3	.016	3	8.426e-3	3	NC	1	NC	1
570			min	-0.003	2	-0.002	9	-0.018	2	-8.532e-3	1	NC	1	NC	1
571	M16A	1	max	0	10	0	3	.005	3	2.497e-3	3	NC	1	NC	1
572			min	0	3	-0.001	1	-0.006	2	-2.516e-3	2	NC	1	NC	1
573		2	max	0	10	0	15	.001	9	2.388e-3	3	NC	1	NC	1
574			min	0	3	-0.003	4	-0.001	2	-2.398e-3	2	NC	1	NC	1
575		3	max	0	10	-0.001	15	.005	1	2.28e-3	3	NC	1	NC	4
576			min	0	3	-0.006	4	-0.004	3	-2.28e-3	2	NC	1	7107.235	3
577		4	max	0	10	-0.002	15	.008	1	2.172e-3	3	NC	3	NC	4
578			min	0	3	-0.009	4	-0.007	3	-2.162e-3	2	7138.056	4	5396.648	3
579		5	max	0	10	-0.003	15	.009	1	2.063e-3	3	NC	5	NC	4
580			min	0	3	-0.011	4	-0.009	3	-2.044e-3	2	5569.897	4	4651.95	3
581		6	max	0	10	-0.003	15	.011	1	1.955e-3	3	NC	5	NC	4
582			min	0	3	-0.013	4	-0.011	3	-1.926e-3	2	4687.657	4	4322.14	3
583		7	max	0	10	-0.004	15	.011	1	1.847e-3	3	NC	5	NC	4
584			min	0	3	-0.015	4	-0.011	3	-1.808e-3	2	4157.106	4	4233.973	3
585		8	max	0	10	-0.004	15	.011	1	1.738e-3	3	NC	5	NC	4
586			min	0	3	-0.016	4	-0.011	3	-1.69e-3	2	3838.696	4	4327.276	3
587		9	max	0	10	-0.004	15	.01	1	1.63e-3	3	NC	5	NC	4
588			min	0	3	-0.017	4	-0.011	3	-1.573e-3	2	3667.309	4	4592.124	3
589		10	max	0	10	-0.004	15	.009	1	1.522e-3	3	NC	15	NC	4
590			min	0	3	-0.017	4	-.01	3	-1.455e-3	2	3613.09	4	5053.686	3
591		11	max	0	10	-0.004	15	.008	1	1.413e-3	3	NC	5	NC	4
592			min	0	3	-0.017	4	-0.009	3	-1.337e-3	2	3667.309	4	5776.981	3
593		12	max	0	10	-0.004	15	.007	1	1.305e-3	3	NC	5	NC	4
594			min	0	3	-0.016	4	-0.007	3	-1.219e-3	2	3838.696	4	6891.073	3
595		13	max	0	10	-0.004	15	.005	1	1.196e-3	3	NC	5	NC	2
596			min	0	3	-0.015	4	-0.006	3	-1.101e-3	2	4157.106	4	8650.991	3
597		14	max	0	10	-0.003	15	.004	1	1.088e-3	3	NC	5	NC	1
598			min	0	3	-0.013	4	-0.004	3	-9.829e-4	2	4687.657	4	NC	1
599		15	max	0	10	-0.003	15	.002	1	9.797e-4	3	NC	5	NC	1
600			min	0	3	-0.011	4	-0.003	3	-8.649e-4	2	5569.897	4	NC	1
601		16	max	0	10	-0.002	15	.001	1	8.713e-4	3	NC	3	NC	1
602			min	0	3	-0.009	4	-0.001	3	-7.47e-4	2	7138.056	4	NC	1
603		17	max	0	10	-0.001	15	0	4	7.63e-4	3	NC	1	NC	1
604			min	0	3	-0.006	4	0	3	-6.291e-4	2	NC	1	NC	1
605		18	max	0	10	0	15	0	4	6.546e-4	3	NC	1	NC	1
606			min	0	3	-0.003	4	0	2	-5.111e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	5.462e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.932e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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Version 2.4.5673.0

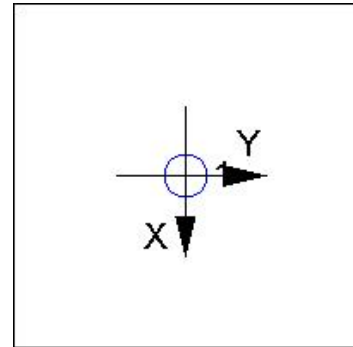
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### 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



### 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

### 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

### 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

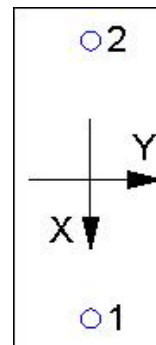
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.