



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.07	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	81 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.476 k-ft
M_z =	0.088 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	61%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.188 k-ft
M_z =	0.000 k-ft
P_n =	-1.010 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	95%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.455 k-ft
P_n =	0.358 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	34%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.012 k-ft
M_z =	0.000 k-ft
P_n =	2.686 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	46%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.012 k-ft
M_z =	0.000 k-ft
P_n =	3.182 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<u>37%</u>



5. FOUNDATION DESIGN CALCULATIONS

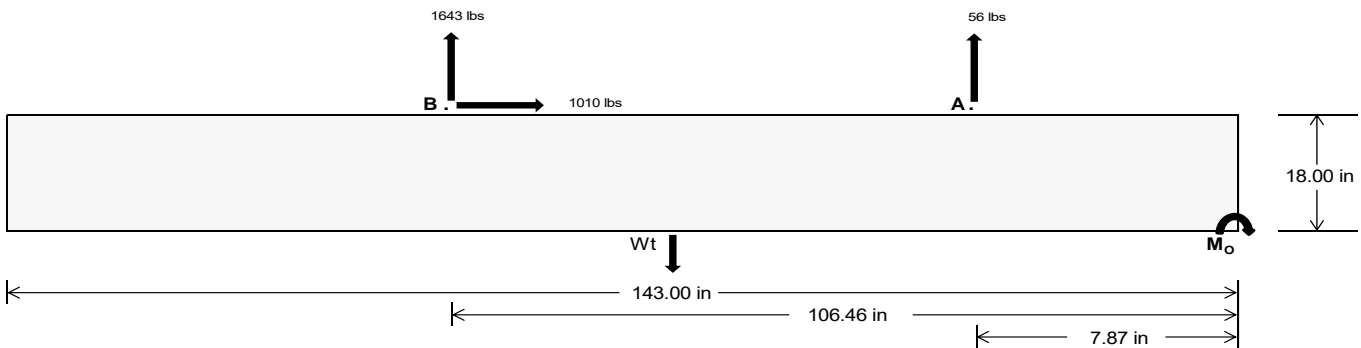
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>242.93</u>	<u>6839.24</u> k
Compressive Load =		<u>3111.44</u>	<u>4975.94</u> k
Lateral Load =		<u>313.01</u>	<u>4200.44</u> k
Moment (Weak Axis) =		<u>0.60</u>	<u>0.20</u> k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 193489.5$ in-lbs
Resisting Force Required = 2706.15 lbs
S.F. = 1.67
Weight Required = 4510.24 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 1010.10 lbs
Friction = 0.4
Weight Required = 2525.25 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 1010.10 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$ 35 in 36 in 37 in 38 in
7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	948 lbs	948 lbs	948 lbs	948 lbs	1314 lbs	1314 lbs	1314 lbs	1314 lbs	1595 lbs	1595 lbs	1595 lbs	1595 lbs	-112 lbs	-112 lbs	-112 lbs	-112 lbs
F_B	918 lbs	918 lbs	918 lbs	918 lbs	2214 lbs	2214 lbs	2214 lbs	2214 lbs	2251 lbs	2251 lbs	2251 lbs	2251 lbs	-3285 lbs	-3285 lbs	-3285 lbs	-3285 lbs
F_V	112 lbs	112 lbs	112 lbs	112 lbs	1809 lbs	1809 lbs	1809 lbs	1809 lbs	1430 lbs	1430 lbs	1430 lbs	1430 lbs	-2020 lbs	-2020 lbs	-2020 lbs	-2020 lbs
P_{total}	9426 lbs	9642 lbs	9858 lbs	10074 lbs	11089 lbs	11305 lbs	11521 lbs	11737 lbs	11406 lbs	11622 lbs	11838 lbs	12054 lbs	1139 lbs	1268 lbs	1398 lbs	1527 lbs
M	2520 lbs-ft	2520 lbs-ft	2520 lbs-ft	2520 lbs-ft	3233 lbs-ft	3233 lbs-ft	3233 lbs-ft	3233 lbs-ft	4043 lbs-ft	4043 lbs-ft	4043 lbs-ft	4043 lbs-ft	5946 lbs-ft	5946 lbs-ft	5946 lbs-ft	5946 lbs-ft
e	0.27 ft	0.26 ft	0.26 ft	0.25 ft	0.29 ft	0.29 ft	0.28 ft	0.28 ft	0.35 ft	0.35 ft	0.34 ft	0.34 ft	5.22 ft	4.69 ft	4.25 ft	3.89 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	234.7 psf	234.2 psf	233.8 psf	233.3 psf	272.2 psf	270.7 psf	269.2 psf	267.9 psf	269.6 psf	268.1 psf	266.8 psf	265.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	307.7 psf	305.2 psf	302.8 psf	300.6 psf	365.9 psf	361.7 psf	357.8 psf	354.1 psf	386.7 psf	382.0 psf	377.6 psf	373.4 psf	353.6 psf	222.0 psf	177.3 psf	155.7 psf

Maximum Bearing Pressure = 387 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

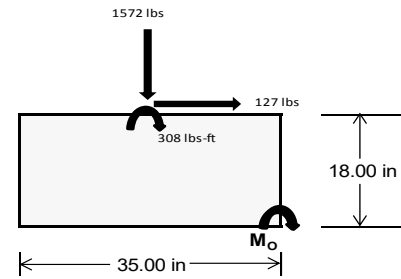
Overturning Check

$M_o = 1794.0 \text{ ft-lbs}$
 Resisting Force Required = 1230.15 lbs
 S.F. = 1.67
 Weight Required = 2050.26 lbs
 Minimum Width = 35 in
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	285 lbs	497 lbs	149 lbs	658 lbs	1572 lbs	554 lbs	131 lbs	145 lbs	-4 lbs
F_h	176 lbs	171 lbs	179 lbs	129 lbs	127 lbs	138 lbs	176 lbs	172 lbs	178 lbs
P_{total}	9644 lbs	9855 lbs	9508 lbs	9567 lbs	10481 lbs	9463 lbs	2868 lbs	2882 lbs	2732 lbs
M	673 lbs-ft	661 lbs-ft	682 lbs-ft	500 lbs-ft	498 lbs-ft	529 lbs-ft	674 lbs-ft	659 lbs-ft	676 lbs-ft
e	0.07 ft	0.07 ft	0.07 ft	0.05 ft	0.05 ft	0.06 ft	0.23 ft	0.23 ft	0.25 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	237.7 psf	244.4 psf	233.2 psf	245.7 psf	272.1 psf	240.9 psf	42.6 psf	43.9 psf	38.6 psf
f_{max}	317.3 psf	322.7 psf	313.9 psf	304.9 psf	331.0 psf	303.6 psf	122.4 psf	121.9 psf	118.6 psf



Maximum Bearing Pressure = 331 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

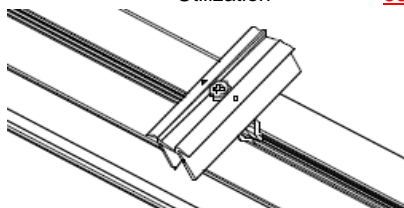
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

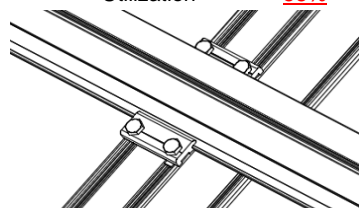
Fastening of Modules to Purlins

Maximum Uplifting Force =	1.045 k
Allowable Uplift =	1.214 k
Utilization =	<u>86%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.528 k
Allowable Uplift =	4.357 k
Utilization =	<u>58%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.393 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>32%</u>

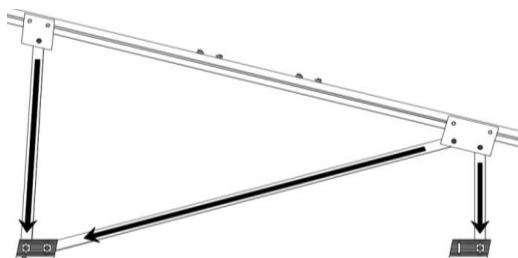
Rear Strut

Maximum Axial Load =	4.607 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>62%</u>

Diagonal Strut

Maximum Axial Load =	2.836 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

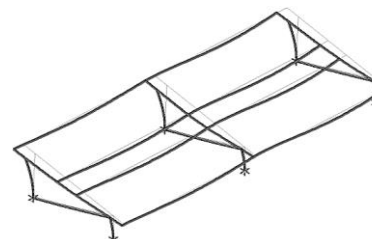
7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	60.93 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.219 in
	<u>0.582 ≤ 1.219, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 81 \text{ in}$$

$$J = 0.432$$

$$224.084$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 81$$

$$J = 0.432$$

$$142.504$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\...\PVMMax 72 Cell 2V 30° 120mph 30psf 6.75ft 7-05Page 19





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-22.384	5	-507.387	2	-41.821	5	-.015	3	-.168	5	0	3
77	M15	1	max	73.768	5	717.897	2	-9.664	12	.017	2	.296	4	0	2
78			min	-32.406	1	-383.655	3	-145.882	1	-.013	3	.013	10	0	3
79		2	max	64.918	5	529.157	2	-8.099	12	.017	2	.212	4	.253	3
80			min	-32.406	1	-289.807	3	-112.656	1	-.013	3	.002	10	-.468	2
81		3	max	56.067	5	340.418	2	-6.533	12	.017	2	.137	5	.435	3
82			min	-32.406	1	-195.959	3	-96.711	4	-.013	3	-.015	1	-.794	2
83		4	max	47.217	5	151.679	2	-3.989	10	.017	2	.08	5	.546	3
84			min	-32.406	1	-102.112	3	-86.643	4	-.013	3	-.062	1	-.978	2
85		5	max	38.367	5	-.259	15	.266	10	.017	2	.025	5	.588	3
86			min	-32.406	1	-37.06	2	-76.575	4	-.013	3	-.084	1	-1.021	2
87		6	max	29.516	5	85.584	3	20.247	1	.017	2	-.005	12	.559	3
88			min	-32.406	1	-225.799	2	-69.981	5	-.013	3	-.082	1	-.923	2
89		7	max	20.666	5	179.431	3	53.473	1	.017	2	-.005	10	.46	3
90			min	-32.406	1	-414.539	2	-67.598	5	-.013	3	-.094	4	-.683	2
91		8	max	11.815	5	273.279	3	86.699	1	.017	2	.005	2	.29	3
92			min	-32.406	1	-603.278	2	-65.215	5	-.013	3	-.133	4	-.301	2
93		9	max	2.965	5	367.127	3	119.925	1	.017	2	.076	1	.222	2
94			min	-32.406	1	-792.017	2	-62.833	5	-.013	3	-.178	5	.002	15
95		10	max	-2.574	10	460.974	3	153.15	1	.013	3	.292	4	.887	2
96			min	-32.406	1	-980.756	2	-119.724	14	-.017	2	-.003	3	-.261	3
97		11	max	-2.574	10	792.017	2	-2.86	12	.013	3	.207	4	.222	2
98			min	-32.406	1	-367.127	3	-119.925	1	-.017	2	-.007	3	.002	15
99		12	max	-2.574	10	603.278	2	-1.295	12	.013	3	.13	4	.29	3
100			min	-32.406	1	-273.279	3	-97.865	4	-.017	2	-.009	3	-.301	2
101		13	max	-2.574	10	414.539	2	.669	3	.013	3	.072	5	.46	3
102			min	-34.53	4	-179.431	3	-87.797	4	-.017	2	-.054	1	-.683	2
103		14	max	-2.574	10	225.799	2	3.017	3	.013	3	.017	5	.559	3
104			min	-43.381	4	-85.584	3	-77.729	4	-.017	2	-.082	1	-.923	2
105		15	max	-2.574	10	37.06	2	12.979	1	.013	3	-.003	12	.588	3
106			min	-52.231	4	.26	15	-70.25	5	-.017	2	-.084	1	-1.021	2
107		16	max	-2.574	10	102.112	3	46.204	1	.013	3	0	3	.546	3
108			min	-61.082	4	-151.679	2	-67.867	5	-.017	2	-.104	4	-.978	2
109		17	max	-2.574	10	195.959	3	79.43	1	.013	3	.007	3	.435	3
110			min	-69.932	4	-340.418	2	-65.484	5	-.017	2	-.143	4	-.794	2
111		18	max	-2.574	10	289.807	3	112.656	1	.013	3	.057	1	.253	3
112			min	-78.782	4	-529.157	2	-63.101	5	-.017	2	-.187	5	-.468	2
113		19	max	-2.574	10	383.655	3	145.882	1	.013	3	.154	1	0	2
114			min	-87.633	4	-717.897	2	-60.718	5	-.017	2	-.233	5	0	3
115	M16	1	max	68.07	5	630.666	2	-8.689	12	.005	1	.207	4	0	2
116			min	-50.655	1	-303.089	3	-139.865	1	-.012	3	.011	10	0	3
117		2	max	59.22	5	441.926	2	-7.124	12	.005	1	.144	4	.192	3
118			min	-50.655	1	-209.242	3	-106.639	1	-.012	3	0	10	-.402	2
119		3	max	50.369	5	253.187	2	-5.558	12	.005	1	.094	5	.314	3
120			min	-50.655	1	-115.394	3	-73.413	1	-.012	3	-.033	1	-.663	2
121		4	max	41.519	5	64.448	2	-3.627	10	.005	1	.056	5	.365	3
122			min	-50.655	1	-21.546	3	-59.676	4	-.012	3	-.076	1	-.782	2
123		5	max	32.669	5	72.302	3	.629	10	.005	1	.019	5	.346	3
124			min	-50.655	1	-124.291	2	-49.608	4	-.012	3	-.094	1	-.76	2
125		6	max	23.818	5	166.149	3	26.265	1	.005	1	-.005	12	.257	3
126			min	-50.655	1	-313.03	2	-44.759	5	-.012	3	-.086	1	-.596	2
127		7	max	14.968	5	259.997	3	59.49	1	.005	1	-.005	10	.097	3
128			min	-50.655	1	-501.77	2	-42.376	5	-.012	3	-.061	4	-.29	2
129		8	max	6.117	5	353.845	3	92.716	1	.005	1	.006	2	.157	2
130			min	-50.655	1	-690.509	2	-39.993	5	-.012	3	-.08	4	-.133	3
131		9	max	-1.776	15	447.692	3	125.942	1	.005	1	.085	1	.746	2
132			min	-50.655	1	-879.248	2	-37.61	5	-.012	3	-.108	5	-.434	3



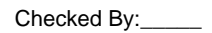
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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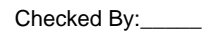
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-4.549	10	541.54	3	159.168	1	.012	3	.207	4	1.476	2
134			min	-50.655	1	-1067.987	2	-110.461	14	-.005	1	.001	12	-.805	3
135		11	max	-3.091	15	879.248	2	-3.835	12	.012	3	.14	4	.746	2
136			min	-50.655	1	-447.692	3	-125.942	1	-.005	1	-.004	3	-.434	3
137		12	max	-4.549	10	690.509	2	-2.27	12	.012	3	.08	4	.157	2
138			min	-50.655	1	-353.845	3	-92.716	1	-.005	1	-.007	3	-.133	3
139		13	max	-4.549	10	501.77	2	-.704	12	.012	3	.04	5	.097	3
140			min	-50.655	1	-259.997	3	-64.718	4	-.005	1	-.054	1	-.29	2
141		14	max	-4.549	10	313.03	2	1.489	3	.012	3	.003	5	.257	3
142			min	-50.655	1	-166.149	3	-54.65	4	-.005	1	-.086	1	-.596	2
143		15	max	-4.549	10	124.291	2	6.961	1	.012	3	-.004	12	.346	3
144			min	-52.816	4	-72.302	3	-45.967	5	-.005	1	-.094	1	-.76	2
145		16	max	-4.549	10	21.546	3	40.187	1	.012	3	-.002	12	.365	3
146			min	-61.667	4	-64.448	2	-43.584	5	-.005	1	-.084	4	-.782	2
147		17	max	-4.549	10	115.394	3	73.413	1	.012	3	.003	3	.314	3
148			min	-70.517	4	-253.187	2	-41.201	5	-.005	1	-.106	4	-.663	2
149		18	max	-4.549	10	209.242	3	106.639	1	.012	3	.034	1	.192	3
150			min	-79.367	4	-441.926	2	-38.818	5	-.005	1	-.128	5	-.402	2
151		19	max	-4.549	10	303.089	3	139.865	1	.012	3	.127	1	0	2
152			min	-88.218	4	-630.666	2	-36.435	5	-.005	1	-.156	5	0	5
153	M2	1	max	1078.184	2	2.062	4	.245	1	0	3	0	3	0	1
154			min	-1516.599	3	.5	15	-20.696	4	0	4	0	2	0	1
155		2	max	1078.713	2	1.991	4	.245	1	0	3	0	1	0	15
156			min	-1516.202	3	.484	15	-21.157	4	0	4	-.007	4	0	4
157		3	max	1079.242	2	1.92	4	.245	1	0	3	0	1	0	15
158			min	-1515.805	3	.467	15	-21.618	4	0	4	-.015	4	-.001	4
159		4	max	1079.772	2	1.849	4	.245	1	0	3	0	1	0	15
160			min	-1515.408	3	.45	15	-22.079	4	0	4	-.023	4	-.002	4
161		5	max	1080.301	2	1.778	4	.245	1	0	3	0	1	0	15
162			min	-1515.011	3	.434	15	-22.541	4	0	4	-.031	4	-.003	4
163		6	max	1080.83	2	1.707	4	.245	1	0	3	0	1	0	15
164			min	-1514.614	3	.417	15	-23.002	4	0	4	-.039	4	-.003	4
165		7	max	1081.36	2	1.635	4	.245	1	0	3	0	1	0	15
166			min	-1514.217	3	.4	15	-23.463	4	0	4	-.048	4	-.004	4
167		8	max	1081.889	2	1.564	4	.245	1	0	3	0	1	-.001	15
168			min	-1513.82	3	.384	15	-23.924	4	0	4	-.056	4	-.005	4
169		9	max	1082.418	2	1.493	4	.245	1	0	3	0	1	-.001	15
170			min	-1513.423	3	.367	15	-24.385	4	0	4	-.065	4	-.005	4
171		10	max	1082.947	2	1.422	4	.245	1	0	3	0	1	-.001	15
172			min	-1513.026	3	.346	12	-24.847	4	0	4	-.074	4	-.006	4
173		11	max	1083.477	2	1.351	4	.245	1	0	3	0	1	-.001	15
174			min	-1512.629	3	.318	12	-25.308	4	0	4	-.083	4	-.006	4
175		12	max	1084.006	2	1.28	4	.245	1	0	3	0	1	-.002	15
176			min	-1512.232	3	.29	12	-25.769	4	0	4	-.092	4	-.007	4
177		13	max	1084.535	2	1.209	4	.245	1	0	3	.001	1	-.002	15
178			min	-1511.835	3	.263	12	-26.23	4	0	4	-.101	4	-.007	4
179		14	max	1085.065	2	1.138	4	.245	1	0	3	.001	1	-.002	15
180			min	-1511.438	3	.235	12	-26.692	4	0	4	-.11	4	-.007	4
181		15	max	1085.594	2	1.067	4	.245	1	0	3	.001	1	-.002	15
182			min	-1511.041	3	.207	12	-27.153	4	0	4	-.12	4	-.008	4
183		16	max	1086.123	2	.996	4	.245	1	0	3	.001	1	-.002	15
184			min	-1510.644	3	.18	12	-27.614	4	0	4	-.13	4	-.008	4
185		17	max	1086.653	2	.925	4	.245	1	0	3	.001	1	-.002	15
186			min	-1510.247	3	.152	12	-28.075	4	0	4	-.14	4	-.009	4
187		18	max	1087.182	2	.864	2	.245	1	0	3	.001	1	-.002	15
188			min	-1509.85	3	.124	12	-28.536	4	0	4	-.15	4	-.009	4
189		19	max	1087.711	2	.809	2	.245	1	0	3	.002	1	-.002	12













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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-139.86	1	-303.246	3	-88.206	4	0	2	-.127	1	-.005	1
533	M5	max	319.712	1	2539.128	3	74.917	5	0	1	0	1	.024	2
534		min	9.168	12	-1443.358	2	0	1	0	4	-.201	4	0	15
535		max	320.554	1	2538.034	3	76.377	5	0	1	0	1	.92	2
536		min	9.589	12	-1444.817	2	0	1	0	4	-.154	4	-1.559	3
537		max	1887.676	3	1411.926	2	59.516	4	0	4	0	1	1.786	2
538		min	-1150.494	2	-1716.414	3	0	1	0	1	-.106	4	-3.088	3
539		max	1888.307	3	1410.467	2	60.977	4	0	4	0	1	.91	2
540		min	-1149.652	2	-1717.509	3	0	1	0	1	-.069	4	-2.022	3
541		max	1888.939	3	1409.008	2	62.437	4	0	4	0	1	.068	1
542		min	-1148.809	2	-1718.603	3	0	1	0	1	-.031	4	-.956	3
543		max	1889.571	3	1407.549	2	63.897	4	0	4	.009	4	.111	3
544		min	-1147.967	2	-1719.697	3	0	1	0	1	0	1	-.839	2
545		max	1890.203	3	1406.089	2	65.357	4	0	4	.049	4	1.179	3
546		min	-1147.125	2	-1720.791	3	0	1	0	1	0	1	-1.712	2
547		max	1890.834	3	1404.63	2	66.817	4	0	4	.09	4	2.247	3
548		min	-1146.282	2	-1721.886	3	0	1	0	1	0	1	-2.584	2
549		max	1903.381	3	178.433	2	179.316	4	0	1	0	1	2.591	3
550		min	-1011.528	2	.438	15	0	1	0	1	-.181	4	-2.968	2
551		max	1904.013	3	176.974	2	180.776	4	0	1	0	1	2.501	3
552		min	-1010.685	2	-.002	15	0	1	0	1	-.069	4	-3.079	2
553		max	1904.645	3	175.515	2	182.236	4	0	1	.043	4	2.411	3
554		min	-1009.843	2	-1.719	6	0	1	0	1	0	1	-3.188	2
555		max	1918.027	3	1114.457	3	189.546	4	0	1	0	1	2.106	3
556		min	-875.463	2	-1759.284	2	0	1	0	4	-.32	4	-2.849	2
557		max	1918.658	3	1113.363	3	191.007	4	0	1	0	1	1.415	3
558		min	-874.621	2	-1760.743	2	0	1	0	4	-.202	4	-1.757	2
559		max	1919.29	3	1112.268	3	192.467	4	0	1	0	1	.724	3
560		min	-873.778	2	-1762.202	2	0	1	0	4	-.083	4	-.663	2
561		max	1919.922	3	1111.174	3	193.927	4	0	1	.037	4	.431	2
562		min	-872.936	2	-1763.661	2	0	1	0	4	0	1	0	15
563		max	1920.554	3	1110.08	3	195.387	4	0	1	.157	4	1.526	2
564		min	-872.093	2	-1765.12	2	0	1	0	4	0	1	-.655	3
565		max	1921.186	3	1108.985	3	196.847	4	0	1	.279	4	2.622	2
566		min	-871.251	2	-1766.579	2	0	1	0	4	0	1	-1.343	3
567		max	-11.222	12	2140.002	2	0	1	0	4	.31	4	1.336	2
568		min	-319.186	1	-1082.344	3	-17.175	5	0	1	0	1	-.696	3
569		max	-10.801	12	2138.543	2	0	1	0	4	.301	4	.01	1
570		min	-318.344	1	-1083.438	3	-15.715	5	0	1	0	1	-.024	3
571	M9	max	139.177	1	776.619	3	56.546	4	0	3	-.01	10	0	15
572		min	9.506	12	-413.563	2	3.57	10	0	4	-.152	4	-.012	2
573		max	140.019	1	775.525	3	58.006	4	0	3	-.008	10	.245	2
574		min	9.927	12	-415.022	2	3.57	10	0	4	-.116	4	-.49	3
575		max	633.854	3	561.958	2	44.294	1	0	2	-.005	10	.493	2
576		min	-380.389	2	-617.689	3	3.552	10	0	3	-.08	4	-.955	3
577		max	634.486	3	560.499	2	44.294	1	0	2	-.003	10	.144	2
578		min	-379.546	2	-618.783	3	3.552	10	0	3	-.055	4	-.572	3
579		max	635.118	3	559.04	2	44.294	1	0	2	-.001	10	-.005	15
580		min	-378.704	2	-619.878	3	3.552	10	0	3	-.029	4	-.203	2
581		max	635.75	3	557.581	2	44.294	1	0	2	.014	1	.198	3
582		min	-377.861	2	-620.972	3	3.552	10	0	3	-.006	5	-.55	2
583		max	636.381	3	556.122	2	45.542	4	0	2	.041	1	.583	3
584		min	-377.019	2	-622.066	3	3.552	10	0	3	.003	10	-.895	2
585		max	637.013	3	554.663	2	47.002	4	0	2	.069	1	.97	3
586		min	-376.177	2	-623.161	3	3.552	10	0	3	.006	10	-1.24	2
587		max	652.823	3	51.999	2	82.591	4	0	3	-.004	10	1.127	3
588		min	-317.248	2	.454	15	6.419	10	0	9	-.133	4	-1.413	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	653.455	3	50.54	2	84.051	4	0	3	0	1	1.105	3
590		min	-316.406	2	.014	15	6.419	10	0	9	-.082	4	-1.445	2
591	11	max	654.086	3	49.081	2	85.511	4	0	3	.047	1	1.084	3
592		min	-315.563	2	-1.724	6	6.419	10	0	9	-.04	5	-1.476	2
593	12	max	669.479	3	424.336	3	154.153	4	0	3	-.006	10	.953	3
594		min	-256.447	2	-666.277	2	3.842	10	0	2	-.255	4	-1.312	2
595	13	max	670.11	3	423.241	3	155.614	4	0	3	-.004	10	.69	3
596		min	-255.605	2	-667.736	2	3.842	10	0	2	-.159	4	-.898	2
597	14	max	670.742	3	422.147	3	157.074	4	0	3	-.001	10	.428	3
598		min	-254.763	2	-669.195	2	3.842	10	0	2	-.061	4	-.483	2
599	15	max	671.374	3	421.053	3	158.534	4	0	3	.036	4	.166	3
600		min	-253.92	2	-670.654	2	3.842	10	0	2	.001	12	-.081	1
601	16	max	672.006	3	419.958	3	159.994	4	0	3	.135	4	.35	2
602		min	-253.078	2	-672.113	2	3.842	10	0	2	.004	10	-.095	3
603	17	max	672.638	3	418.864	3	161.454	4	0	3	.235	4	.767	2
604		min	-252.235	2	-673.572	2	3.842	10	0	2	.006	10	-.355	3
605	18	max	-9.111	12	632.905	2	50.71	1	0	2	.243	4	.388	2
606		min	-140.703	1	-302.152	3	-69.73	5	0	3	.009	10	-.176	3
607	19	max	-8.69	12	631.446	2	50.71	1	0	2	.207	4	.012	3
608		min	-139.86	1	-303.246	3	-68.27	5	0	3	.011	10	-.005	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.224	2	.012	3	1.549e-2	2	NC	1	NC	1
2			min	-605	4	-.073	3	-.008	2	-5.08e-3	3	NC	1	NC	1
3		2	max	0	1	.179	2	.014	3	1.627e-2	2	NC	4	NC	1
4			min	-605	4	.005	15	-.009	5	-4.548e-3	3	1308.537	3	NC	1
5		3	max	0	1	.153	3	.025	1	1.706e-2	2	NC	4	NC	2
6			min	-605	4	.004	15	-.012	5	-4.015e-3	3	716.68	3	6175.913	1
7		4	max	0	1	.219	3	.036	1	1.784e-2	2	NC	4	NC	2
8			min	-605	4	.003	15	-.01	5	-3.482e-3	3	554.539	3	4270.905	1
9		5	max	0	1	.242	3	.041	1	1.863e-2	2	NC	4	NC	2
10			min	-605	4	.003	15	-.004	10	-2.949e-3	3	514.113	3	3775.312	1
11		6	max	0	1	.222	3	.038	1	1.941e-2	2	NC	4	NC	2
12			min	-605	4	.004	15	-.006	10	-2.417e-3	3	548.049	3	4087.849	1
13		7	max	0	1	.211	2	.032	3	2.02e-2	2	NC	2	NC	2
14			min	-606	4	.004	15	-.009	10	-1.884e-3	3	669.838	3	5627.981	1
15		8	max	0	1	.262	2	.034	3	2.098e-2	2	NC	4	NC	1
16			min	-606	4	.005	15	-.014	2	-1.351e-3	3	954.438	3	7464.171	3
17		9	max	0	1	.307	2	.035	3	2.177e-2	2	NC	4	NC	1
18			min	-606	4	.006	15	-.021	2	-8.186e-4	3	1577.106	3	7176.196	3
19		10	max	0	1	.326	2	.035	3	2.255e-2	2	NC	4	NC	1
20			min	-606	4	-.001	3	-.025	2	-2.859e-4	3	1583.001	2	7105.981	3
21		11	max	0	10	.307	2	.035	3	2.177e-2	2	NC	4	NC	1
22			min	-606	4	.006	15	-.021	2	-8.186e-4	3	1577.106	3	7176.196	3
23		12	max	0	10	.262	2	.034	3	2.098e-2	2	NC	4	NC	1
24			min	-606	4	.005	15	-.014	2	-1.351e-3	3	954.438	3	7464.171	3
25		13	max	0	10	.211	2	.032	3	2.02e-2	2	NC	2	NC	2
26			min	-606	4	.004	15	-.009	10	-1.884e-3	3	669.838	3	5627.981	1
27		14	max	0	10	.222	3	.038	1	1.941e-2	2	NC	4	NC	2
28			min	-606	4	.003	15	-.006	10	-2.417e-3	3	548.049	3	4087.849	1
29		15	max	0	10	.242	3	.041	1	1.863e-2	2	NC	4	NC	2
30			min	-606	4	.002	15	-.004	10	-2.949e-3	3	514.113	3	3775.312	1
31		16	max	0	10	.219	3	.036	1	1.784e-2	2	NC	4	NC	2
32			min	-606	4	.002	15	-.003	10	-3.482e-3	3	554.539	3	4270.905	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.063	1	.032	4	1.336e-2	3	NC	4	NC	2
148			min	-.115	4	-.128	3	-.002	10	-1.357e-2	2	1080.06	2	5031.018	4
149		18	max	0	1	.115	2	.021	4	1.257e-2	3	NC	4	NC	1
150			min	-.115	4	-.147	3	-.003	10	-1.33e-2	2	1935.264	2	7536.523	4
151		19	max	0	1	.199	2	.009	3	1.178e-2	3	NC	1	NC	1
152			min	-.115	4	-.174	3	-.006	2	-1.303e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.012	2	.006	1	1.937e-3	5	NC	1	NC	1
154			min	-.011	3	-.018	3	-.57	4	-1.264e-4	1	6424.542	2	135.961	4
155		2	max	.008	2	.01	2	.005	1	1.955e-3	5	NC	1	NC	1
156			min	-.011	3	-.018	3	-.524	4	-1.202e-4	1	7447.873	2	147.931	4
157		3	max	.007	2	.009	2	.005	1	1.972e-3	5	NC	1	NC	1
158			min	-.01	3	-.017	3	-.478	4	-1.141e-4	1	8837.852	2	162.128	4
159		4	max	.007	2	.007	2	.004	1	1.99e-3	5	NC	1	NC	1
160			min	-.009	3	-.017	3	-.433	4	-1.079e-4	1	NC	1	179.133	4
161		5	max	.006	2	.006	2	.004	1	2.007e-3	5	NC	1	NC	1
162			min	-.009	3	-.016	3	-.388	4	-1.018e-4	1	NC	1	199.733	4
163		6	max	.006	2	.004	2	.003	1	2.025e-3	5	NC	1	NC	1
164			min	-.008	3	-.015	3	-.344	4	-9.559e-5	1	NC	1	225.017	4
165		7	max	.005	2	.003	2	.003	1	2.043e-3	5	NC	1	NC	1
166			min	-.008	3	-.014	3	-.302	4	-8.943e-5	1	NC	1	256.532	4
167		8	max	.005	2	.002	2	.003	1	2.06e-3	5	NC	1	NC	1
168			min	-.007	3	-.014	3	-.261	4	-8.327e-5	1	NC	1	296.53	4
169		9	max	.004	2	0	2	.002	1	2.078e-3	5	NC	1	NC	1
170			min	-.006	3	-.013	3	-.222	4	-7.711e-5	1	NC	1	348.397	4
171		10	max	.004	2	0	2	.002	1	2.096e-3	4	NC	1	NC	1
172			min	-.006	3	-.012	3	-.186	4	-7.095e-5	1	NC	1	417.418	4
173		11	max	.004	2	-.001	15	.001	1	2.116e-3	4	NC	1	NC	1
174			min	-.005	3	-.011	3	-.151	4	-6.478e-5	1	NC	1	512.227	4
175		12	max	.003	2	-.001	15	.001	1	2.135e-3	4	NC	1	NC	1
176			min	-.004	3	-.01	3	-.12	4	-5.862e-5	1	NC	1	647.739	4
177		13	max	.003	2	-.001	15	0	1	2.154e-3	4	NC	1	NC	1
178			min	-.004	3	-.009	3	-.091	4	-5.246e-5	1	NC	1	851.58	4
179		14	max	.002	2	-.001	15	0	1	2.174e-3	4	NC	1	NC	1
180			min	-.003	3	-.008	3	-.066	4	-4.63e-5	1	NC	1	1179.826	4
181		15	max	.002	2	-.001	15	0	1	2.193e-3	4	NC	1	NC	1
182			min	-.003	3	-.006	3	-.044	4	-4.014e-5	1	NC	1	1761.362	4
183		16	max	.001	2	0	15	0	1	2.213e-3	4	NC	1	NC	1
184			min	-.002	3	-.005	3	-.026	4	-3.398e-5	1	NC	1	2951.046	4
185		17	max	0	2	0	15	0	1	2.232e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	3	-.013	4	-2.782e-5	1	NC	1	6056.566	4
187		18	max	0	2	0	15	0	1	2.251e-3	4	NC	1	NC	1
188			min	0	3	-.002	6	-.004	4	-2.166e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.271e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.55e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	3.155e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.914e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.012	4	5.418e-5	4	NC	1	NC	1
194			min	0	2	-.003	6	0	1	1.605e-6	10	NC	1	8367.15	4
195		3	max	.001	3	-.001	15	.023	4	5.997e-4	4	NC	1	NC	1
196			min	0	2	-.006	6	0	1	2.985e-6	10	NC	1	4350.671	4
197		4	max	.002	3	-.002	15	.034	4	1.145e-3	4	NC	1	NC	1
198			min	-.001	2	-.009	6	0	1	4.365e-6	10	NC	1	3016.213	4
199		5	max	.002	3	-.003	15	.043	4	1.691e-3	4	NC	1	NC	1
200			min	-.002	2	-.012	6	0	1	5.745e-6	10	8545.303	6	2351.353	4
201		6	max	.003	3	-.003	15	.052	4	2.236e-3	4	NC	2	NC	1
202			min	-.002	2	-.015	6	0	1	7.125e-6	10	6922.868	6	1953.351	4
203		7	max	.003	3	-.004	15	.06	4	2.782e-3	4	NC	5	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.003	2	-.017	6	0	1	8.505e-6	10	5945.799	6	1687.719	4
205		8	max	.004	3	-.004	15	.068	4	3.327e-3	4	NC	5	NC	1
206			min	-.003	2	-.019	6	0	1	9.885e-6	10	5343.166	6	1496.595	4
207		9	max	.004	3	-.004	15	.075	4	3.873e-3	4	NC	5	NC	1
208			min	-.004	2	-.02	6	0	3	1.126e-5	10	4987.448	6	1350.851	4
209		10	max	.005	3	-.005	15	.082	4	4.418e-3	4	NC	5	NC	1
210			min	-.004	2	-.021	6	0	12	1.264e-5	10	4816.811	6	1234.148	4
211		11	max	.005	3	-.005	15	.089	4	4.964e-3	4	NC	5	NC	1
212			min	-.005	2	-.021	6	0	12	1.402e-5	10	4806.16	6	1136.576	4
213		12	max	.006	3	-.004	15	.097	4	5.51e-3	4	NC	5	NC	1
214			min	-.005	2	-.02	6	0	12	1.54e-5	10	4957.521	6	1051.792	4
215		13	max	.006	3	-.004	15	.104	4	6.055e-3	4	NC	5	NC	1
216			min	-.006	2	-.019	6	0	12	1.678e-5	10	5302.057	6	975.602	4
217		14	max	.007	3	-.004	15	.112	4	6.601e-3	4	NC	5	NC	1
218			min	-.006	2	-.017	6	0	12	1.816e-5	10	5916.161	6	905.217	4
219		15	max	.007	3	-.003	15	.121	4	7.146e-3	4	NC	3	NC	1
220			min	-.006	2	-.014	6	0	12	1.954e-5	10	6968.94	6	838.831	4
221		16	max	.008	3	-.002	15	.131	4	7.692e-3	4	NC	1	NC	1
222			min	-.007	2	-.011	6	0	12	2.092e-5	10	8869.795	6	775.354	4
223		17	max	.009	3	-.001	15	.142	4	8.237e-3	4	NC	1	NC	1
224			min	-.007	2	-.008	6	0	10	2.23e-5	10	NC	1	714.237	4
225		18	max	.009	3	0	15	.155	4	8.783e-3	4	NC	1	NC	1
226			min	-.008	2	-.005	3	0	10	2.368e-5	10	NC	1	655.316	4
227		19	max	.01	3	0	5	.17	4	9.328e-3	4	NC	1	NC	1
228			min	-.008	2	-.002	3	0	10	2.506e-5	10	NC	1	598.694	4
229	M4	1	max	.002	1	.008	2	0	10	4.234e-4	4	NC	1	NC	2
230			min	0	5	-.01	3	-.17	4	9.205e-6	10	NC	1	146.077	4
231		2	max	.002	1	.008	2	0	10	4.234e-4	4	NC	1	NC	2
232			min	0	5	-.009	3	-.156	4	9.205e-6	10	NC	1	158.718	4
233		3	max	.002	1	.007	2	0	10	4.234e-4	4	NC	1	NC	2
234			min	0	5	-.009	3	-.143	4	9.205e-6	10	NC	1	173.769	4
235		4	max	.002	1	.007	2	0	10	4.234e-4	4	NC	1	NC	2
236			min	0	5	-.008	3	-.129	4	9.205e-6	10	NC	1	191.855	4
237		5	max	.002	1	.006	2	0	10	4.234e-4	4	NC	1	NC	2
238			min	0	5	-.008	3	-.116	4	9.205e-6	10	NC	1	213.824	4
239		6	max	.002	1	.006	2	0	10	4.234e-4	4	NC	1	NC	2
240			min	0	5	-.007	3	-.103	4	9.205e-6	10	NC	1	240.855	4
241		7	max	.001	1	.005	2	0	10	4.234e-4	4	NC	1	NC	2
242			min	0	5	-.007	3	-.09	4	9.205e-6	10	NC	1	274.619	4
243		8	max	.001	1	.005	2	0	10	4.234e-4	4	NC	1	NC	2
244			min	0	5	-.006	3	-.078	4	9.205e-6	10	NC	1	317.552	4
245		9	max	.001	1	.004	2	0	10	4.234e-4	4	NC	1	NC	1
246			min	0	5	-.005	3	-.066	4	9.205e-6	10	NC	1	373.323	4
247		10	max	.001	1	.004	2	0	10	4.234e-4	4	NC	1	NC	1
248			min	0	5	-.005	3	-.055	4	9.205e-6	10	NC	1	447.668	4
249		11	max	0	1	.004	2	0	10	4.234e-4	4	NC	1	NC	1
250			min	0	5	-.004	3	-.045	4	9.205e-6	10	NC	1	549.965	4
251		12	max	0	1	.003	2	0	10	4.234e-4	4	NC	1	NC	1
252			min	0	5	-.004	3	-.036	4	9.205e-6	10	NC	1	696.437	4
253		13	max	0	1	.003	2	0	10	4.234e-4	4	NC	1	NC	1
254			min	0	5	-.003	3	-.027	4	9.205e-6	10	NC	1	917.188	4
255		14	max	0	1	.002	2	0	10	4.234e-4	4	NC	1	NC	1
256			min	0	5	-.003	3	-.019	4	9.205e-6	10	NC	1	1273.446	4
257		15	max	0	1	.002	2	0	10	4.234e-4	4	NC	1	NC	1
258			min	0	5	-.002	3	-.013	4	9.205e-6	10	NC	1	1906.317	4
259		16	max	0	1	.001	2	0	10	4.234e-4	4	NC	1	NC	1
260			min	0	5	-.002	3	-.008	4	9.205e-6	10	NC	1	3205.722	4



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	10	4.234e-4	4	NC	1	NC	1
262			min	0	5	-.001	3	-.004	4	9.205e-6	10	NC	1	6616.651	4
263		18	max	0	1	0	2	0	10	4.234e-4	4	NC	1	NC	1
264			min	0	5	0	3	-.001	4	9.205e-6	10	NC	1	NC	1
265		19	max	0	1	0	1	0	1	4.234e-4	4	NC	1	NC	1
266			min	0	1	0	1	0	1	9.205e-6	10	NC	1	NC	1
267	M6	1	max	.024	2	.038	2	0	1	2.015e-3	4	NC	3	NC	1
268			min	-.034	3	-.054	3	-.576	4	0	1	2048.045	2	134.627	4
269		2	max	.022	2	.034	2	0	1	2.032e-3	4	NC	3	NC	1
270			min	-.032	3	-.051	3	-.529	4	0	1	2249.694	2	146.48	4
271		3	max	.021	2	.031	2	0	1	2.049e-3	4	NC	3	NC	1
272			min	-.031	3	-.048	3	-.483	4	0	1	2493.257	2	160.539	4
273		4	max	.02	2	.028	2	0	1	2.065e-3	4	NC	3	NC	1
274			min	-.029	3	-.045	3	-.437	4	0	1	2790.684	2	177.377	4
275		5	max	.018	2	.025	2	0	1	2.082e-3	4	NC	3	NC	1
276			min	-.027	3	-.042	3	-.392	4	0	1	3158.648	2	197.775	4
277		6	max	.017	2	.021	2	0	1	2.098e-3	4	NC	3	NC	1
278			min	-.025	3	-.039	3	-.348	4	0	1	3620.99	2	222.812	4
279		7	max	.016	2	.018	2	0	1	2.115e-3	4	NC	1	NC	1
280			min	-.023	3	-.036	3	-.305	4	0	1	4212.772	2	254.019	4
281		8	max	.014	2	.016	2	0	1	2.132e-3	4	NC	1	NC	1
282			min	-.021	3	-.033	3	-.264	4	0	1	4987.293	2	293.623	4
283		9	max	.013	2	.013	2	0	1	2.148e-3	4	NC	1	NC	1
284			min	-.019	3	-.03	3	-.225	4	0	1	6028.876	2	344.979	4
285		10	max	.012	2	.01	2	0	1	2.165e-3	4	NC	1	NC	1
286			min	-.017	3	-.027	3	-.187	4	0	1	7477.541	2	413.316	4
287		11	max	.011	2	.008	2	0	1	2.182e-3	4	NC	1	NC	1
288			min	-.015	3	-.024	3	-.153	4	0	1	9580.26	2	507.183	4
289		12	max	.009	2	.006	2	0	1	2.198e-3	4	NC	1	NC	1
290			min	-.013	3	-.021	3	-.121	4	0	1	NC	1	641.339	4
291		13	max	.008	2	.004	2	0	1	2.215e-3	4	NC	1	NC	1
292			min	-.011	3	-.018	3	-.092	4	0	1	NC	1	843.121	4
293		14	max	.007	2	.003	2	0	1	2.232e-3	4	NC	1	NC	1
294			min	-.01	3	-.015	3	-.066	4	0	1	NC	1	1168.013	4
295		15	max	.005	2	.002	2	0	1	2.248e-3	4	NC	1	NC	1
296			min	-.008	3	-.012	3	-.044	4	0	1	NC	1	1743.504	4
297		16	max	.004	2	0	2	0	1	2.265e-3	4	NC	1	NC	1
298			min	-.006	3	-.009	3	-.027	4	0	1	NC	1	2920.498	4
299		17	max	.003	2	0	2	0	1	2.281e-3	4	NC	1	NC	1
300			min	-.004	3	-.006	3	-.013	4	0	1	NC	1	5991.425	4
301		18	max	.001	2	0	2	0	1	2.298e-3	4	NC	1	NC	1
302			min	-.002	3	-.003	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.315e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.016e-4	4	NC	1	NC	1
307		2	max	.002	3	0	15	.012	4	2.815e-5	4	NC	1	NC	1
308			min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309		3	max	.003	3	-.001	15	.024	4	5.579e-4	4	NC	1	NC	1
310			min	-.003	2	-.008	3	0	1	0	1	NC	1	NC	1
311		4	max	.005	3	-.002	15	.034	4	1.088e-3	4	NC	1	NC	1
312			min	-.004	2	-.011	3	0	1	0	1	NC	1	NC	1
313		5	max	.006	3	-.003	15	.044	4	1.617e-3	4	NC	1	NC	1
314			min	-.006	2	-.014	3	0	1	0	1	8051.538	3	NC	1
315		6	max	.008	3	-.004	15	.053	4	2.147e-3	4	NC	1	NC	1
316			min	-.007	2	-.017	3	0	1	0	1	6794.734	3	NC	1
317		7	max	.009	3	-.004	15	.061	4	2.677e-3	4	NC	2	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.009	2	-.019	3	0	1	0	1	5921.482	4	NC	1
319	8	max	.011	3	-.005	15	.069	4	3.206e-3	4	NC	2	NC	1
320		min	-.01	2	-.021	3	0	1	0	1	5322.859	4	NC	1
321	9	max	.012	3	-.005	15	.076	4	3.736e-3	4	NC	5	NC	1
322		min	-.012	2	-.022	3	0	1	0	1	4969.695	4	NC	1
323	10	max	.014	3	-.005	15	.083	4	4.266e-3	4	NC	5	NC	1
324		min	-.013	2	-.023	3	0	1	0	1	4800.64	4	NC	1
325	11	max	.015	3	-.005	15	.09	4	4.796e-3	4	NC	5	NC	1
326		min	-.015	2	-.023	3	0	1	0	1	4790.846	4	NC	1
327	12	max	.017	3	-.005	15	.097	4	5.325e-3	4	NC	5	NC	1
328		min	-.016	2	-.022	3	0	1	0	1	4942.439	4	NC	1
329	13	max	.018	3	-.005	15	.104	4	5.855e-3	4	NC	5	NC	1
330		min	-.018	2	-.021	3	0	1	0	1	5286.569	4	NC	1
331	14	max	.02	3	-.004	15	.111	4	6.385e-3	4	NC	2	NC	1
332		min	-.019	2	-.019	3	0	1	0	1	5899.476	4	NC	1
333	15	max	.022	3	-.004	15	.119	4	6.914e-3	4	NC	1	NC	1
334		min	-.02	2	-.017	3	0	1	0	1	6949.86	4	NC	1
335	16	max	.023	3	-.003	15	.128	4	7.444e-3	4	NC	1	NC	1
336		min	-.022	2	-.015	3	0	1	0	1	8846.086	4	NC	1
337	17	max	.025	3	-.002	15	.139	4	7.974e-3	4	NC	1	NC	1
338		min	-.023	2	-.012	3	0	1	0	1	NC	1	NC	1
339	18	max	.026	3	-.001	15	.15	4	8.504e-3	4	NC	1	NC	1
340		min	-.025	2	-.01	3	0	1	0	1	NC	1	NC	1
341	19	max	.028	3	0	10	.163	4	9.033e-3	4	NC	1	NC	1
342		min	-.026	2	-.007	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	2	.026	2	0	2.425e-4	4	NC	1	NC	1
344		min	0	3	-.028	3	-.163	4	0	1	NC	1	151.784	4
345	2	max	.005	2	.024	2	0	1	2.425e-4	4	NC	1	NC	1
346		min	0	3	-.027	3	-.15	4	0	1	NC	1	164.94	4
347	3	max	.005	2	.023	2	0	1	2.425e-4	4	NC	1	NC	1
348		min	0	3	-.025	3	-.137	4	0	1	NC	1	180.602	4
349	4	max	.005	2	.021	2	0	1	2.425e-4	4	NC	1	NC	1
350		min	0	3	-.024	3	-.124	4	0	1	NC	1	199.422	4
351	5	max	.004	2	.02	2	0	1	2.425e-4	4	NC	1	NC	1
352		min	0	3	-.022	3	-.112	4	0	1	NC	1	222.282	4
353	6	max	.004	2	.019	2	0	1	2.425e-4	4	NC	1	NC	1
354		min	0	3	-.021	3	-.099	4	0	1	NC	1	250.407	4
355	7	max	.004	2	.017	2	0	1	2.425e-4	4	NC	1	NC	1
356		min	0	3	-.019	3	-.087	4	0	1	NC	1	285.537	4
357	8	max	.003	2	.016	2	0	1	2.425e-4	4	NC	1	NC	1
358		min	0	3	-.017	3	-.075	4	0	1	NC	1	330.207	4
359	9	max	.003	2	.014	2	0	1	2.425e-4	4	NC	1	NC	1
360		min	0	3	-.016	3	-.064	4	0	1	NC	1	388.236	4
361	10	max	.003	2	.013	2	0	1	2.425e-4	4	NC	1	NC	1
362		min	0	3	-.014	3	-.053	4	0	1	NC	1	465.589	4
363	11	max	.003	2	.011	2	0	1	2.425e-4	4	NC	1	NC	1
364		min	0	3	-.013	3	-.043	4	0	1	NC	1	572.027	4
365	12	max	.002	2	.01	2	0	1	2.425e-4	4	NC	1	NC	1
366		min	0	3	-.011	3	-.034	4	0	1	NC	1	724.432	4
367	13	max	.002	2	.009	2	0	1	2.425e-4	4	NC	1	NC	1
368		min	0	3	-.009	3	-.026	4	0	1	NC	1	954.128	4
369	14	max	.002	2	.007	2	0	1	2.425e-4	4	NC	1	NC	1
370		min	0	3	-.008	3	-.019	4	0	1	NC	1	1324.833	4
371	15	max	.001	2	.006	2	0	1	2.425e-4	4	NC	1	NC	1
372		min	0	3	-.006	3	-.013	4	0	1	NC	1	1983.39	4
373	16	max	0	2	.004	2	0	1	2.425e-4	4	NC	1	NC	1
374		min	0	3	-.005	3	-.007	4	0	1	NC	1	3335.594	4



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.003	2	0	1	2.425e-4	4	NC	1	NC	1
376			min	0	3	-.003	3	-.004	4	0	1	NC	1	6885.342	4
377		18	max	0	2	.001	2	0	1	2.425e-4	4	NC	1	NC	1
378			min	0	3	-.002	3	-.001	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	2.425e-4	4	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	2	.012	2	0	10	2.002e-3	4	NC	1	NC	1
382			min	-.011	3	-.018	3	-.574	4	9.237e-6	10	6424.542	2	135.095	4
383		2	max	.008	2	.01	2	0	10	2.018e-3	4	NC	1	NC	1
384			min	-.011	3	-.018	3	-.527	4	8.786e-6	10	7447.873	2	146.99	4
385		3	max	.007	2	.009	2	0	10	2.033e-3	4	NC	1	NC	1
386			min	-.01	3	-.017	3	-.481	4	8.336e-6	10	8837.852	2	161.098	4
387		4	max	.007	2	.007	2	0	10	2.049e-3	4	NC	1	NC	1
388			min	-.009	3	-.017	3	-.435	4	7.885e-6	10	NC	1	177.997	4
389		5	max	.006	2	.006	2	0	10	2.064e-3	4	NC	1	NC	1
390			min	-.009	3	-.016	3	-.39	4	7.435e-6	10	NC	1	198.469	4
391		6	max	.006	2	.004	2	0	10	2.08e-3	4	NC	1	NC	1
392			min	-.008	3	-.015	3	-.347	4	6.984e-6	10	NC	1	223.597	4
393		7	max	.005	2	.003	2	0	10	2.095e-3	4	NC	1	NC	1
394			min	-.008	3	-.014	3	-.304	4	6.534e-6	10	NC	1	254.918	4
395		8	max	.005	2	.002	2	0	10	2.11e-3	4	NC	1	NC	1
396			min	-.007	3	-.014	3	-.263	4	6.083e-6	10	NC	1	294.669	4
397		9	max	.004	2	0	2	0	10	2.126e-3	4	NC	1	NC	1
398			min	-.006	3	-.013	3	-.224	4	5.633e-6	10	NC	1	346.217	4
399		10	max	.004	2	0	2	0	10	2.141e-3	4	NC	1	NC	1
400			min	-.006	3	-.012	3	-.187	4	5.182e-6	10	NC	1	414.815	4
401		11	max	.004	2	-.001	2	0	10	2.157e-3	4	NC	1	NC	1
402			min	-.005	3	-.011	3	-.152	4	4.732e-6	10	NC	1	509.046	4
403		12	max	.003	2	-.002	2	0	10	2.172e-3	4	NC	1	NC	1
404			min	-.004	3	-.01	3	-.12	4	4.281e-6	10	NC	1	643.734	4
405		13	max	.003	2	-.002	15	0	10	2.188e-3	4	NC	1	NC	1
406			min	-.004	3	-.009	3	-.092	4	3.831e-6	10	NC	1	846.342	4
407		14	max	.002	2	-.002	15	0	10	2.203e-3	4	NC	1	NC	1
408			min	-.003	3	-.008	3	-.066	4	3.38e-6	10	NC	1	1172.612	4
409		15	max	.002	2	-.002	15	0	10	2.219e-3	4	NC	1	NC	1
410			min	-.003	3	-.006	3	-.044	4	2.93e-6	10	NC	1	1750.676	4
411		16	max	.001	2	-.001	15	0	10	2.234e-3	4	NC	1	NC	1
412			min	-.002	3	-.005	4	-.026	4	2.479e-6	10	NC	1	2933.331	4
413		17	max	0	2	0	15	0	10	2.249e-3	4	NC	1	NC	1
414			min	-.001	3	-.004	4	-.013	4	2.028e-6	10	NC	1	6020.828	4
415		18	max	0	2	0	15	0	10	2.265e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.004	4	1.578e-6	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.28e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	1.127e-6	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-2.256e-7	10	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.933e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.012	4	4.535e-5	5	NC	1	NC	1
422			min	0	2	-.003	4	0	10	-1.801e-5	1	NC	1	8332.663	4
423		3	max	.001	3	-.002	15	.023	4	5.791e-4	5	NC	1	NC	1
424			min	0	2	-.006	4	0	10	-3.286e-5	1	NC	1	4333.794	4
425		4	max	.002	3	-.002	15	.034	4	1.115e-3	4	NC	1	NC	1
426			min	-.001	2	-.009	4	0	10	-4.772e-5	1	NC	1	3005.635	4
427		5	max	.002	3	-.003	15	.043	4	1.651e-3	4	NC	1	NC	1
428			min	-.002	2	-.013	4	0	10	-6.257e-5	1	8241.942	4	2344.323	4
429		6	max	.003	3	-.004	15	.052	4	2.188e-3	4	NC	2	NC	1
430			min	-.002	2	-.016	4	0	10	-7.743e-5	1	6698.279	4	1948.836	4
431		7	max	.003	3	-.004	15	.06	4	2.724e-3	4	NC	5	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.003	2	-.018	4	0	10	-9.228e-5	1	5767.752	4	1685.261	4
433		8	max	.004	3	-.005	15	.068	4	3.26e-3	4	NC	5	NC	1
434			min	-.003	2	-.02	4	0	10	-1.071e-4	1	5194.206	4	1495.978	4
435		9	max	.004	3	-.005	15	.075	4	3.796e-3	4	NC	5	NC	1
436			min	-.004	2	-.022	4	0	1	-1.22e-4	1	4857.023	4	1351.973	4
437		10	max	.005	3	-.006	15	.082	4	4.332e-3	4	NC	5	NC	1
438			min	-.004	2	-.022	4	0	1	-1.368e-4	1	4697.859	4	1236.958	4
439		11	max	.005	3	-.006	15	.089	4	4.868e-3	4	NC	5	NC	1
440			min	-.005	2	-.022	4	0	1	-1.517e-4	1	4693.385	4	1141.034	4
441		12	max	.006	3	-.005	15	.096	4	5.405e-3	4	NC	5	NC	1
442			min	-.005	2	-.022	4	0	1	-1.666e-4	1	4846.355	4	1057.844	4
443		13	max	.006	3	-.005	15	.103	4	5.941e-3	4	NC	5	NC	1
444			min	-.006	2	-.021	4	-.001	1	-1.814e-4	1	5187.812	4	983.162	4
445		14	max	.007	3	-.005	15	.111	4	6.477e-3	4	NC	5	NC	1
446			min	-.006	2	-.019	4	-.002	1	-1.963e-4	1	5793.006	4	914.152	4
447		15	max	.007	3	-.004	15	.12	4	7.013e-3	4	NC	3	NC	1
448			min	-.006	2	-.016	4	-.002	1	-2.111e-4	1	6828.031	4	848.959	4
449		16	max	.008	3	-.003	15	.129	4	7.549e-3	4	NC	1	NC	1
450			min	-.007	2	-.013	4	-.003	1	-2.26e-4	1	8694.625	4	786.447	4
451		17	max	.009	3	-.002	15	.14	4	8.085e-3	4	NC	1	NC	1
452			min	-.007	2	-.009	4	-.004	1	-2.408e-4	1	NC	1	726.028	4
453		18	max	.009	3	-.002	15	.152	4	8.622e-3	4	NC	1	NC	1
454			min	-.008	2	-.005	4	-.005	1	-2.557e-4	1	NC	1	667.519	4
455		19	max	.01	3	0	10	.166	4	9.158e-3	4	NC	1	NC	1
456			min	-.008	2	-.002	3	-.005	1	-2.705e-4	1	NC	1	611.023	4
457	M12	1	max	.002	1	.008	2	.005	1	3.752e-4	5	NC	1	NC	2
458			min	0	3	-.01	3	-.166	4	-1.048e-4	1	NC	1	149.086	4
459		2	max	.002	1	.008	2	.005	1	3.752e-4	5	NC	1	NC	2
460			min	0	3	-.009	3	-.153	4	-1.048e-4	1	NC	1	161.992	4
461		3	max	.002	1	.007	2	.005	1	3.752e-4	5	NC	1	NC	2
462			min	0	3	-.009	3	-.14	4	-1.048e-4	1	NC	1	177.358	4
463		4	max	.002	1	.007	2	.004	1	3.752e-4	5	NC	1	NC	2
464			min	0	3	-.008	3	-.127	4	-1.048e-4	1	NC	1	195.823	4
465		5	max	.002	1	.006	2	.004	1	3.752e-4	5	NC	1	NC	2
466			min	0	3	-.008	3	-.114	4	-1.048e-4	1	NC	1	218.253	4
467		6	max	.002	1	.006	2	.003	1	3.752e-4	5	NC	1	NC	2
468			min	0	3	-.007	3	-.101	4	-1.048e-4	1	NC	1	245.849	4
469		7	max	.001	1	.005	2	.003	1	3.752e-4	5	NC	1	NC	2
470			min	0	3	-.007	3	-.088	4	-1.048e-4	1	NC	1	280.319	4
471		8	max	.001	1	.005	2	.003	1	3.752e-4	5	NC	1	NC	2
472			min	0	3	-.006	3	-.077	4	-1.048e-4	1	NC	1	324.15	4
473		9	max	.001	1	.004	2	.002	1	3.752e-4	5	NC	1	NC	1
474			min	0	3	-.005	3	-.065	4	-1.048e-4	1	NC	1	381.089	4
475		10	max	.001	1	.004	2	.002	1	3.752e-4	5	NC	1	NC	1
476			min	0	3	-.005	3	-.054	4	-1.048e-4	1	NC	1	456.989	4
477		11	max	0	1	.004	2	.001	1	3.752e-4	5	NC	1	NC	1
478			min	0	3	-.004	3	-.044	4	-1.048e-4	1	NC	1	561.427	4
479		12	max	0	1	.003	2	.001	1	3.752e-4	5	NC	1	NC	1
480			min	0	3	-.004	3	-.035	4	-1.048e-4	1	NC	1	710.964	4
481		13	max	0	1	.003	2	0	1	3.752e-4	5	NC	1	NC	1
482			min	0	3	-.003	3	-.026	4	-1.048e-4	1	NC	1	936.336	4
483		14	max	0	1	.002	2	0	1	3.752e-4	5	NC	1	NC	1
484			min	0	3	-.003	3	-.019	4	-1.048e-4	1	NC	1	1300.055	4
485		15	max	0	1	.002	2	0	1	3.752e-4	5	NC	1	NC	1
486			min	0	3	-.002	3	-.013	4	-1.048e-4	1	NC	1	1946.183	4
487		16	max	0	1	.001	2	0	1	3.752e-4	5	NC	1	NC	1
488			min	0	3	-.002	3	-.008	4	-1.048e-4	1	NC	1	3272.822	4



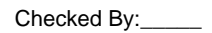
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	3.752e-4	5	NC	1	NC	1
490			min	0	3	-.001	3	-.004	4	-1.048e-4	1	NC	1	6755.294	4
491		18	max	0	1	0	2	0	1	3.752e-4	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-1.048e-4	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	3.752e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-1.048e-4	1	NC	1	NC	1
495	M1	1	max	.012	3	.224	2	.606	4	5.296e-3	2	NC	1	NC	1
496			min	-.008	2	-.073	3	0	10	-1.425e-2	3	NC	1	NC	1
497		2	max	.012	3	.108	2	.588	4	5.728e-3	4	NC	5	NC	1
498			min	-.008	2	-.034	3	-.004	1	-7.079e-3	3	1170.508	2	NC	1
499		3	max	.012	3	.019	3	.57	4	1.054e-2	4	NC	5	NC	1
500			min	-.008	2	-.015	2	-.006	1	-9.996e-5	1	568.38	2	6797.482	5
501		4	max	.012	3	.096	3	.55	4	9.03e-3	4	NC	5	NC	1
502			min	-.008	2	-.149	2	-.005	1	-3.732e-3	3	363.164	2	4992.289	5
503		5	max	.012	3	.189	3	.529	4	7.517e-3	4	NC	15	NC	1
504			min	-.007	2	-.288	2	-.004	1	-7.369e-3	3	264.724	2	4080.267	5
505		6	max	.011	3	.287	3	.507	4	1.072e-2	2	9349.7	15	NC	1
506			min	-.007	2	-.421	2	-.002	1	-1.101e-2	3	210.1	2	3512.294	5
507		7	max	.011	3	.379	3	.486	4	1.43e-2	2	7920.158	15	NC	1
508			min	-.007	2	-.539	2	0	3	-1.464e-2	3	177.67	2	3085.062	4
509		8	max	.011	3	.455	3	.464	4	1.787e-2	2	7071.762	15	NC	1
510			min	-.007	2	-.632	2	0	10	-1.828e-2	3	158.411	2	2738.548	4
511		9	max	.011	3	.505	3	.442	4	1.996e-2	2	6626.566	15	NC	1
512			min	-.007	2	-.691	2	0	1	-1.892e-2	3	148.349	2	2479.441	4
513		10	max	.01	3	.523	3	.417	4	2.107e-2	2	6490.09	15	NC	1
514			min	-.007	2	-.71	2	0	10	-1.756e-2	3	145.411	2	2377.001	4
515		11	max	.01	3	.511	3	.39	4	2.218e-2	2	6626.047	15	NC	1
516			min	-.007	2	-.69	2	0	10	-1.62e-2	3	148.912	2	2381.458	4
517		12	max	.01	3	.469	3	.36	4	2.117e-2	2	7070.534	15	NC	1
518			min	-.006	2	-.629	2	0	1	-1.424e-2	3	160.04	2	2485.233	4
519		13	max	.01	3	.4	3	.325	4	1.697e-2	2	7917.791	15	NC	1
520			min	-.006	2	-.531	2	0	1	-1.139e-2	3	181.469	2	2891.729	4
521		14	max	.009	3	.312	3	.286	4	1.278e-2	2	9345.423	15	NC	1
522			min	-.006	2	-.409	2	0	12	-8.546e-3	3	217.975	2	3848.325	4
523		15	max	.009	3	.212	3	.246	4	8.581e-3	2	NC	15	NC	1
524			min	-.006	2	-.273	2	0	12	-5.698e-3	3	280.485	2	6162.856	4
525		16	max	.009	3	.108	3	.207	4	7.457e-3	4	NC	5	NC	1
526			min	-.006	2	-.136	2	0	10	-2.85e-3	3	395.411	2	NC	1
527		17	max	.009	3	.007	3	.17	4	8.614e-3	4	NC	5	NC	1
528			min	-.006	2	-.008	2	0	10	-2.71e-6	3	639.029	2	NC	1
529		18	max	.009	3	.101	2	.14	4	4.87e-3	2	NC	5	NC	1
530			min	-.006	2	-.086	3	0	10	-1.585e-3	3	1347.154	2	NC	1
531		19	max	.009	3	.199	2	.115	4	9.709e-3	2	NC	1	NC	1
532			min	-.006	2	-.174	3	0	1	-3.246e-3	3	NC	1	NC	1
533	M5	1	max	.035	3	.326	2	.606	4	0	1	NC	1	NC	1
534			min	-.025	2	-.001	3	0	1	-1.537e-5	4	NC	1	NC	1
535		2	max	.035	3	.155	2	.593	4	5.392e-3	4	NC	5	NC	1
536			min	-.025	2	.003	15	0	1	0	1	803.127	2	9347.108	4
537		3	max	.035	3	.055	3	.575	4	1.067e-2	4	NC	5	NC	1
538			min	-.025	2	-.041	2	0	1	0	1	373.076	2	5557.762	4
539		4	max	.034	3	.183	3	.555	4	8.693e-3	4	NC	15	NC	1
540			min	-.024	2	-.282	2	0	1	0	1	224.838	2	4365.313	4
541		5	max	.033	3	.366	3	.532	4	6.716e-3	4	7735.907	15	NC	1
542			min	-.024	2	-.548	2	0	1	0	1	156.187	2	3802.356	4
543		6	max	.033	3	.576	3	.508	4	4.739e-3	4	5908.656	15	NC	1
544			min	-.024	2	-.815	2	0	1	0	1	119.549	2	3446.23	4
545		7	max	.032	3	.783	3	.485	4	2.762e-3	4	4862.23	15	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.009	3	.007	3	.167	4	8.4e-3	4	NC	5	NC	1
604		min	-.006	2	-.008	2	-.005	1	-3.733e-4	1	639.029	2	NC	1
605	18	max	.009	3	.101	2	.138	4	4.148e-3	5	NC	5	NC	1
606		min	-.006	2	-.086	3	-.004	1	-4.87e-3	2	1347.154	2	NC	1
607	19	max	.009	3	.199	2	.116	4	3.246e-3	3	NC	1	NC	1
608		min	-.006	2	-.174	3	0	10	-9.709e-3	2	NC	1	NC	1



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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

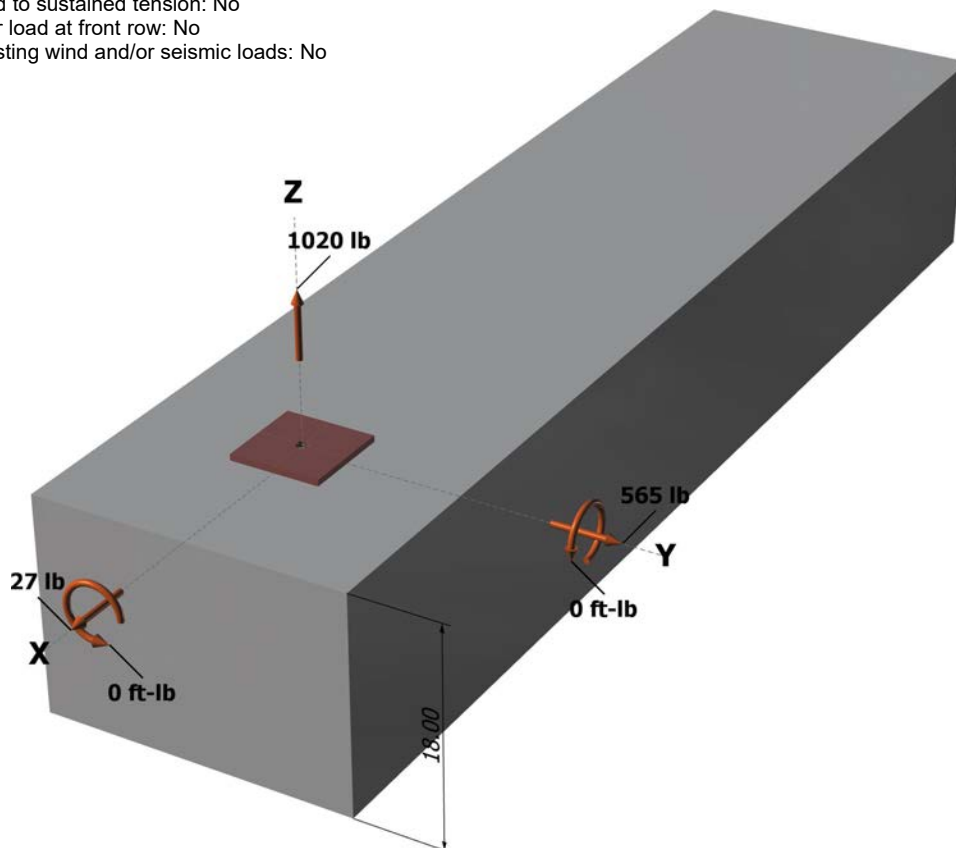
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (ϵ_o): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbv} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbv} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

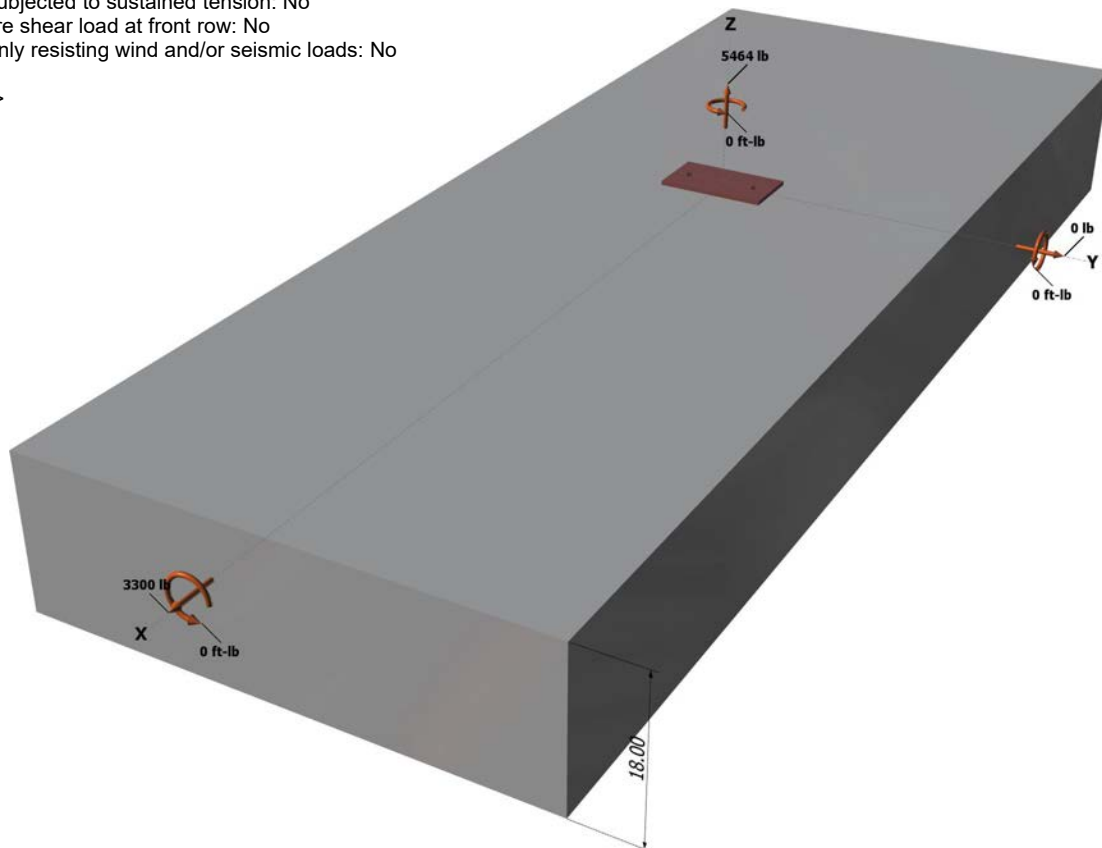
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



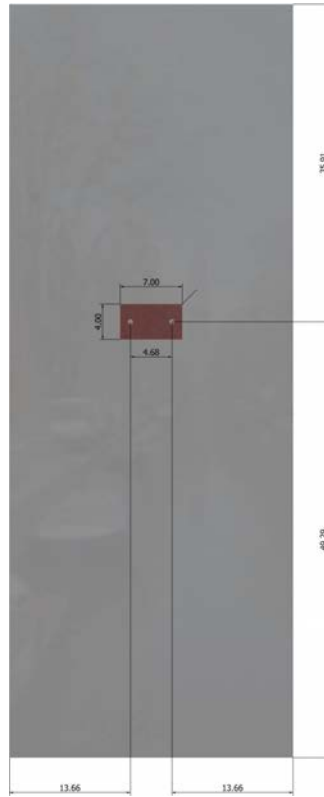
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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Engineer:	HCV	Page:	3/5
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Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

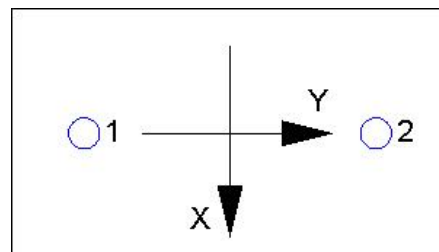
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag} ; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
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Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.