



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	140 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 30.77 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.030 k-ft
P_n =	0.151 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.766 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.640 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	14%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.188 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

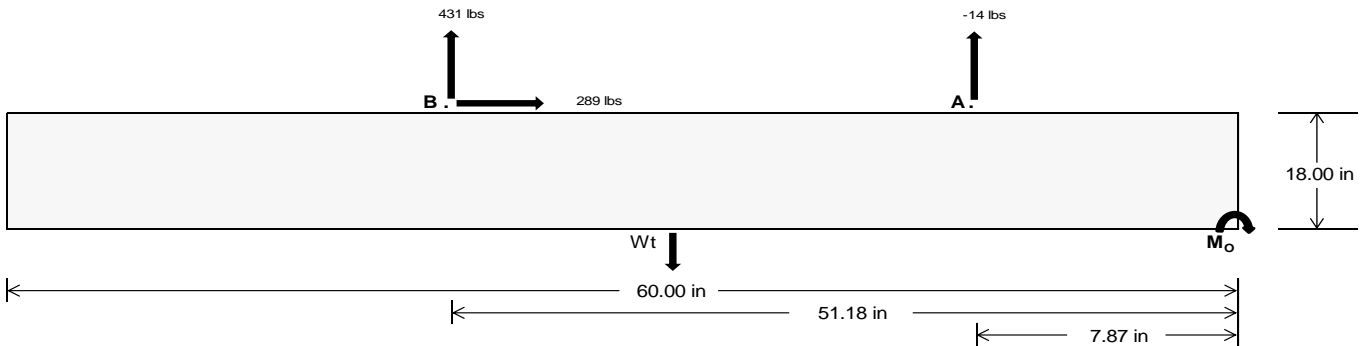
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	5.36	1868.84	k
Compressive Load =	926.13	1246.42	k
Lateral Load =	24.61	1254.29	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27132.9$ in-lbs
Resisting Force Required = 904.43 lbs
S.F. = 1.67
Weight Required = 1507.38 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 289.32 lbs
Friction = 0.4
Weight Required = 723.29 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 289.32 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	326 lbs	326 lbs	326 lbs	326 lbs	327 lbs	327 lbs	327 lbs	327 lbs	457 lbs	457 lbs	457 lbs	457 lbs	28 lbs	28 lbs	28 lbs	28 lbs
F_B	213 lbs	213 lbs	213 lbs	213 lbs	537 lbs	537 lbs	537 lbs	537 lbs	540 lbs	540 lbs	540 lbs	540 lbs	-861 lbs	-861 lbs	-861 lbs	-861 lbs
F_V	35 lbs	35 lbs	35 lbs	35 lbs	523 lbs	523 lbs	523 lbs	523 lbs	415 lbs	415 lbs	415 lbs	415 lbs	-579 lbs	-579 lbs	-579 lbs	-579 lbs
P_{total}	2442 lbs	2533 lbs	2623 lbs	2714 lbs	2768 lbs	2859 lbs	2949 lbs	3040 lbs	2900 lbs	2991 lbs	3081 lbs	3172 lbs	309 lbs	363 lbs	417 lbs	472 lbs
M	279 lbs-ft	279 lbs-ft	279 lbs-ft	279 lbs-ft	439 lbs-ft	439 lbs-ft	439 lbs-ft	439 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	703 lbs-ft	703 lbs-ft	703 lbs-ft	703 lbs-ft
e	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.28 ft	1.94 ft	1.68 ft	1.49 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	240.8 psf	239.8 psf	238.8 psf	237.9 psf	256.1 psf	254.4 psf	252.7 psf	251.3 psf	261.2 psf	259.2 psf	257.4 psf	255.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	317.4 psf	312.8 psf	308.7 psf	304.9 psf	376.6 psf	369.3 psf	362.7 psf	356.7 psf	401.7 psf	393.3 psf	385.6 psf	378.6 psf	530.1 psf	234.4 psf	178.0 psf	155.8 psf

Maximum Bearing Pressure = 530 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

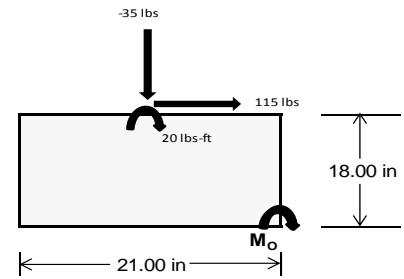
Overturning Check

$M_o = 222.0$ ft-lbs
 Resisting Force Required = 253.69 lbs
 S.F. = 1.67
 Weight Required = 422.82 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	127 lbs	64 lbs	62 lbs	219 lbs	396 lbs	169 lbs	86 lbs	-35 lbs	23 lbs
F_v	14 lbs	115 lbs	14 lbs	10 lbs	86 lbs	11 lbs	14 lbs	115 lbs	14 lbs
P_{total}	2483 lbs	2420 lbs	2418 lbs	2462 lbs	2639 lbs	2412 lbs	775 lbs	654 lbs	712 lbs
M	39 lbs-ft	192 lbs-ft	41 lbs-ft	28 lbs-ft	144 lbs-ft	31 lbs-ft	39 lbs-ft	192 lbs-ft	41 lbs-ft
e	0.02 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.29 ft	0.06 ft
$L/6$	0.29 ft	1.59 ft	1.72 ft	1.73 ft	1.64 ft	1.72 ft	1.65 ft	1.16 ft	1.63 ft
f_{min}	268.5 sqft	201.4 sqft	260.2 sqft	270.4 sqft	244.9 sqft	263.4 sqft	73.1 sqft	-0.4 sqft	65.2 sqft
f_{max}	299.1 psf	351.8 psf	292.4 psf	292.2 psf	358.2 psf	287.9 psf	103.9 psf	149.9 psf	97.4 psf



Maximum Bearing Pressure = 358 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.694 k
Allowable Uplift =	1.214 k
Utilization =	<u>57%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.118 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.712 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

Diagonal Strut

Maximum Axial Load =	0.766 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



Rear Strut

Maximum Axial Load =	1.138 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.188 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.062 in
	<u>0.062 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.22$$

$$22.2924$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$L_b = 33.78 \text{ in}$$

$$r_y = 1.374$$

$$C_b = 1.22$$

$$24.5845$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.453 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	203.556	2	-.031	15	.151	1	0	10	0	4	0	15
30			min	-370.292	3	-.128	4	-.572	5	0	4	0	3	0	6
31		16	max	203.691	2	-.044	15	.151	1	0	10	0	4	0	15
32			min	-370.19	3	-.186	4	-.695	5	0	4	0	3	0	6
33		17	max	203.826	2	-.058	15	.151	1	0	10	0	4	0	15
34			min	-370.089	3	-.243	4	-.818	5	0	4	0	3	0	6
35		18	max	203.961	2	-.071	15	.151	1	0	10	0	1	0	15
36			min	-369.988	3	-.301	4	-.941	5	0	4	0	3	0	6
37		19	max	204.096	2	-.085	15	.151	1	0	10	0	1	0	15
38			min	-369.887	3	-.358	4	-1.064	5	0	4	0	3	0	6
39	M3	1	max	229.694	2	1.734	6	-.008	10	0	5	0	1	0	6
40			min	-221.587	3	.407	15	-1.33	4	0	1	0	10	0	15
41		2	max	229.624	2	1.558	6	-.008	10	0	5	0	1	0	2
42			min	-221.64	3	.366	15	-1.197	4	0	1	0	10	0	3
43		3	max	229.554	2	1.381	6	-.008	10	0	5	0	1	0	2
44			min	-221.692	3	.324	15	-1.063	4	0	1	0	5	0	3
45		4	max	229.484	2	1.205	6	-.008	10	0	5	0	1	0	15
46			min	-221.745	3	.283	15	-.929	4	0	1	0	5	0	4
47		5	max	229.414	2	1.029	6	-.008	10	0	5	0	1	0	15
48			min	-221.797	3	.241	15	-.796	4	0	1	0	5	0	4
49		6	max	229.344	2	.852	6	-.008	10	0	5	0	1	0	15
50			min	-221.85	3	.2	15	-.662	4	0	1	0	5	0	4
51		7	max	229.274	2	.676	6	-.008	10	0	5	0	1	0	15
52			min	-221.902	3	.158	15	-.528	4	0	1	0	5	0	4
53		8	max	229.204	2	.499	6	-.008	10	0	5	0	1	0	15
54			min	-221.955	3	.117	15	-.395	4	0	1	0	5	-.001	4
55		9	max	229.134	2	.323	6	-.008	10	0	5	0	1	0	15
56			min	-222.007	3	.075	15	-.261	4	0	1	0	5	-.001	4
57		10	max	229.064	2	.147	6	-.008	10	0	5	0	1	0	15
58			min	-222.06	3	.034	15	-.185	1	0	1	0	5	-.001	4
59		11	max	228.994	2	.006	2	.052	5	0	5	0	1	0	15
60			min	-222.112	3	-.054	3	-.185	1	0	1	0	5	-.001	4
61		12	max	228.924	2	-.049	15	.185	5	0	5	0	1	0	15
62			min	-222.165	3	-.206	4	-.185	1	0	1	0	5	-.001	4
63		13	max	228.854	2	-.091	15	.319	5	0	5	0	1	0	15
64			min	-222.217	3	-.382	4	-.185	1	0	1	0	5	-.001	4
65		14	max	228.784	2	-.132	15	.453	5	0	5	0	1	0	15
66			min	-222.27	3	-.559	4	-.185	1	0	1	0	5	-.001	4
67		15	max	228.714	2	-.173	15	.586	5	0	5	0	1	0	15
68			min	-222.322	3	-.735	4	-.185	1	0	1	0	5	0	4
69		16	max	228.644	2	-.215	15	.72	5	0	5	0	1	0	15
70			min	-222.375	3	-.912	4	-.185	1	0	1	0	5	0	4
71		17	max	228.574	2	-.256	15	.854	5	0	5	0	10	0	15
72			min	-222.427	3	-1.088	4	-.185	1	0	1	0	4	0	4
73		18	max	228.504	2	-.298	15	.987	5	0	5	0	10	0	15
74			min	-222.48	3	-1.264	4	-.185	1	0	1	0	4	0	4
75		19	max	228.434	2	-.339	15	1.121	5	0	5	0	5	0	1
76			min	-222.532	3	-1.441	4	-.185	1	0	1	0	1	0	1
77	M4	1	max	268.521	1	0	1	-.03	10	0	1	0	5	0	1
78			min	21.55	15	0	1	-17.823	4	0	1	0	2	0	1
79		2	max	268.586	1	0	1	-.03	10	0	1	0	10	0	1
80			min	21.569	15	0	1	-17.88	4	0	1	-.002	4	0	1
81		3	max	268.65	1	0	1	-.03	10	0	1	0	10	0	1
82			min	21.589	15	0	1	-17.936	4	0	1	-.003	4	0	1
83		4	max	268.715	1	0	1	-.03	10	0	1	0	10	0	1
84			min	21.608	15	0	1	-17.992	4	0	1	-.005	4	0	1
85		5	max	268.78	1	0	1	-.03	10	0	1	0	10	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	21.628	15	0	1	-18.048	4	0	1	-.006	4	0	1
87		6	max	268.844	1	0	1	-.03	10	0	1	0	10	0	1
88			min	21.647	15	0	1	-18.104	4	0	1	-.008	4	0	1
89		7	max	268.909	1	0	1	-.03	10	0	1	0	10	0	1
90			min	21.667	15	0	1	-18.16	4	0	1	-.01	4	0	1
91		8	max	268.974	1	0	1	-.03	10	0	1	0	10	0	1
92			min	21.687	15	0	1	-18.216	4	0	1	-.011	4	0	1
93		9	max	269.039	1	0	1	-.03	10	0	1	0	10	0	1
94			min	21.706	15	0	1	-18.272	4	0	1	-.013	4	0	1
95		10	max	269.103	1	0	1	-.03	10	0	1	0	10	0	1
96			min	21.726	15	0	1	-18.328	4	0	1	-.015	4	0	1
97		11	max	269.168	1	0	1	-.03	10	0	1	0	10	0	1
98			min	21.745	15	0	1	-18.384	4	0	1	-.016	4	0	1
99		12	max	269.233	1	0	1	-.03	10	0	1	0	10	0	1
100			min	21.765	15	0	1	-18.44	4	0	1	-.018	4	0	1
101		13	max	269.297	1	0	1	-.03	10	0	1	0	10	0	1
102			min	21.784	15	0	1	-18.496	4	0	1	-.019	4	0	1
103		14	max	269.362	1	0	1	-.03	10	0	1	0	10	0	1
104			min	21.804	15	0	1	-18.552	4	0	1	-.021	4	0	1
105		15	max	269.427	1	0	1	-.03	10	0	1	0	10	0	1
106			min	21.823	15	0	1	-18.609	4	0	1	-.023	4	0	1
107		16	max	269.492	1	0	1	-.03	10	0	1	0	10	0	1
108			min	21.843	15	0	1	-18.665	4	0	1	-.024	4	0	1
109		17	max	269.556	1	0	1	-.03	10	0	1	0	10	0	1
110			min	21.862	15	0	1	-18.721	4	0	1	-.026	4	0	1
111		18	max	269.621	1	0	1	-.03	10	0	1	0	10	0	1
112			min	21.882	15	0	1	-18.777	4	0	1	-.028	4	0	1
113		19	max	269.686	1	0	1	-.03	10	0	1	0	10	0	1
114			min	21.901	15	0	1	-18.833	4	0	1	-.029	4	0	1
115	M6	1	max	637.602	2	.66	6	1.118	4	0	3	0	3	0	1
116			min	-1137.862	3	.146	15	-.25	3	0	5	0	2	0	1
117		2	max	637.737	2	.603	6	.994	4	0	3	0	3	0	15
118			min	-1137.761	3	.132	15	-.25	3	0	5	0	2	0	6
119		3	max	637.872	2	.545	6	.871	4	0	3	0	4	0	15
120			min	-1137.66	3	.119	15	-.25	3	0	5	0	2	0	6
121		4	max	638.007	2	.491	2	.748	4	0	3	0	4	0	15
122			min	-1137.559	3	.105	15	-.25	3	0	5	0	2	0	6
123		5	max	638.142	2	.446	2	.625	4	0	3	0	4	0	15
124			min	-1137.458	3	.092	15	-.25	3	0	5	0	2	0	6
125		6	max	638.276	2	.401	2	.502	4	0	3	0	4	0	15
126			min	-1137.356	3	.077	12	-.25	3	0	5	0	2	0	6
127		7	max	638.411	2	.357	2	.379	4	0	3	0	4	0	15
128			min	-1137.255	3	.054	12	-.25	3	0	5	0	2	0	2
129		8	max	638.546	2	.312	2	.255	4	0	3	.001	4	0	15
130			min	-1137.154	3	.032	12	-.25	3	0	5	0	3	0	2
131		9	max	638.681	2	.267	2	.132	4	0	3	.001	4	0	15
132			min	-1137.053	3	.002	3	-.25	3	0	5	0	3	0	2
133		10	max	638.816	2	.222	2	.03	9	0	3	.001	4	0	15
134			min	-1136.952	3	-.032	3	-.25	3	0	5	0	3	0	2
135		11	max	638.951	2	.177	2	.03	9	0	3	.001	4	0	12
136			min	-1136.851	3	-.065	3	-.25	3	0	5	0	3	0	2
137		12	max	639.086	2	.133	2	.03	9	0	3	.001	4	0	12
138			min	-1136.75	3	-.099	3	-.25	3	0	5	0	3	0	2
139		13	max	639.22	2	.088	2	.03	9	0	3	0	4	0	12
140			min	-1136.648	3	-.133	3	-.373	5	0	5	0	3	0	2
141		14	max	639.355	2	.043	2	.03	9	0	3	0	4	0	12
142			min	-1136.547	3	-.166	3	-.496	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	639.49	2	-0.002	2	.03	9	0	3	0	4	0	12
144			min	-1136.446	3	-.2	3	-.619	5	0	5	0	3	0	2
145		16	max	639.625	2	-.047	2	.03	9	0	3	0	4	0	3
146			min	-1136.345	3	-.233	3	-.742	5	0	5	0	3	0	2
147		17	max	639.76	2	-.07	15	.03	9	0	3	0	4	0	3
148			min	-1136.244	3	-.267	3	-.866	5	0	5	0	3	0	2
149		18	max	639.895	2	-.084	15	.03	9	0	3	0	4	0	3
150			min	-1136.143	3	-.318	4	-.989	5	0	5	0	3	0	2
151		19	max	640.03	2	-.097	15	.03	9	0	3	0	14	0	3
152			min	-1136.042	3	-.375	4	-1.112	5	0	5	0	3	0	2
153	M7	1	max	766.468	2	1.757	4	.048	3	0	1	0	4	0	2
154			min	-654.73	3	.421	15	-1.298	4	0	3	0	3	0	3
155		2	max	766.398	2	1.581	4	.048	3	0	1	0	4	0	2
156			min	-654.783	3	.38	15	-1.164	4	0	3	0	3	0	3
157		3	max	766.328	2	1.405	4	.048	3	0	1	0	1	0	2
158			min	-654.835	3	.338	15	-1.03	4	0	3	0	3	0	3
159		4	max	766.258	2	1.228	4	.048	3	0	1	0	1	0	2
160			min	-654.888	3	.297	15	-.897	4	0	3	0	3	0	3
161		5	max	766.188	2	1.052	4	.048	3	0	1	0	1	0	15
162			min	-654.94	3	.255	15	-.763	4	0	3	0	5	0	3
163		6	max	766.118	2	.875	4	.048	3	0	1	0	1	0	15
164			min	-654.993	3	.214	15	-.629	4	0	3	0	5	0	3
165		7	max	766.048	2	.699	4	.048	3	0	1	0	1	0	15
166			min	-655.045	3	.172	15	-.496	4	0	3	0	5	0	6
167		8	max	765.978	2	.523	4	.048	3	0	1	0	1	0	15
168			min	-655.098	3	.128	12	-.362	4	0	3	0	5	-.001	6
169		9	max	765.908	2	.346	4	.048	3	0	1	0	1	0	15
170			min	-655.15	3	.059	12	-.228	4	0	3	0	5	-.001	6
171		10	max	765.838	2	.205	2	.048	3	0	1	0	1	0	15
172			min	-655.203	3	-.026	3	-.095	4	0	3	-.001	5	-.001	6
173		11	max	765.768	2	.067	2	.048	3	0	1	0	1	0	15
174			min	-655.255	3	-.129	3	-.011	1	0	3	-.001	5	-.001	6
175		12	max	765.698	2	-.035	15	.173	5	0	1	0	1	0	15
176			min	-655.308	3	-.232	3	-.011	1	0	3	0	5	-.001	6
177		13	max	765.628	2	-.076	15	.307	5	0	1	0	1	0	15
178			min	-655.36	3	-.36	6	-.011	1	0	3	0	5	-.001	6
179		14	max	765.558	2	-.118	15	.44	5	0	1	0	1	0	15
180			min	-655.413	3	-.536	6	-.011	1	0	3	0	5	-.001	6
181		15	max	765.488	2	-.159	15	.574	5	0	1	0	1	0	15
182			min	-655.465	3	-.713	6	-.011	1	0	3	0	5	0	6
183		16	max	765.418	2	-.201	15	.708	5	0	1	0	1	0	15
184			min	-655.518	3	-.889	6	-.011	1	0	3	0	5	0	6
185		17	max	765.348	2	-.242	15	.841	5	0	1	0	1	0	15
186			min	-655.57	3	-1.065	6	-.011	1	0	3	0	5	0	6
187		18	max	765.278	2	-.284	15	.975	5	0	1	0	1	0	15
188			min	-655.623	3	-1.242	6	-.011	1	0	3	0	3	0	6
189		19	max	765.208	2	-.325	15	1.109	5	0	1	0	1	0	1
190			min	-655.675	3	-1.418	6	-.011	1	0	3	0	3	0	1
191	M8	1	max	711.241	1	0	1	.243	1	0	1	0	4	0	1
192			min	21.561	15	0	1	-18.053	4	0	1	0	3	0	1
193		2	max	711.306	1	0	1	.243	1	0	1	0	1	0	1
194			min	21.58	15	0	1	-18.109	4	0	1	-.002	4	0	1
195		3	max	711.371	1	0	1	.243	1	0	1	0	1	0	1
196			min	21.6	15	0	1	-18.165	4	0	1	-.003	4	0	1
197		4	max	711.435	1	0	1	.243	1	0	1	0	1	0	1
198			min	21.619	15	0	1	-18.221	4	0	1	-.005	4	0	1
199		5	max	711.5	1	0	1	.243	1	0	1	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	21.639	15	0	1	-18.278	4	0	1	-.006	4	0	1
201		6	max	711.565	1	0	1	.243	1	0	1	0	1	0	1
202			min	21.658	15	0	1	-18.334	4	0	1	-.008	4	0	1
203		7	max	711.629	1	0	1	.243	1	0	1	0	1	0	1
204			min	21.678	15	0	1	-18.39	4	0	1	-.01	4	0	1
205		8	max	711.694	1	0	1	.243	1	0	1	0	1	0	1
206			min	21.697	15	0	1	-18.446	4	0	1	-.011	4	0	1
207		9	max	711.759	1	0	1	.243	1	0	1	0	1	0	1
208			min	21.717	15	0	1	-18.502	4	0	1	-.013	4	0	1
209		10	max	711.823	1	0	1	.243	1	0	1	0	1	0	1
210			min	21.737	15	0	1	-18.558	4	0	1	-.015	4	0	1
211		11	max	711.888	1	0	1	.243	1	0	1	0	1	0	1
212			min	21.756	15	0	1	-18.614	4	0	1	-.016	4	0	1
213		12	max	711.953	1	0	1	.243	1	0	1	0	1	0	1
214			min	21.776	15	0	1	-18.67	4	0	1	-.018	4	0	1
215		13	max	712.018	1	0	1	.243	1	0	1	0	1	0	1
216			min	21.795	15	0	1	-18.726	4	0	1	-.02	4	0	1
217		14	max	712.082	1	0	1	.243	1	0	1	0	1	0	1
218			min	21.815	15	0	1	-18.782	4	0	1	-.021	4	0	1
219		15	max	712.147	1	0	1	.243	1	0	1	0	1	0	1
220			min	21.834	15	0	1	-18.838	4	0	1	-.023	4	0	1
221		16	max	712.212	1	0	1	.243	1	0	1	0	1	0	1
222			min	21.854	15	0	1	-18.894	4	0	1	-.025	4	0	1
223		17	max	712.276	1	0	1	.243	1	0	1	0	1	0	1
224			min	21.873	15	0	1	-18.95	4	0	1	-.026	4	0	1
225		18	max	712.341	1	0	1	.243	1	0	1	0	1	0	1
226			min	21.893	15	0	1	-19.007	4	0	1	-.028	4	0	1
227		19	max	712.406	1	0	1	.243	1	0	1	0	1	0	1
228			min	21.912	15	0	1	-19.063	4	0	1	-.03	4	0	1
229	M10	1	max	203.023	2	.711	4	1.238	5	0	1	0	1	0	1
230			min	-302.046	3	.181	15	-.116	1	-.001	5	0	3	0	1
231		2	max	203.158	2	.653	4	1.115	5	0	1	0	1	0	15
232			min	-301.944	3	.168	15	-.116	1	-.001	5	0	3	0	4
233		3	max	203.292	2	.596	4	.992	5	0	1	0	4	0	15
234			min	-301.843	3	.154	15	-.116	1	-.001	5	0	3	0	4
235		4	max	203.427	2	.538	4	.869	5	0	1	0	4	0	15
236			min	-301.742	3	.141	15	-.116	1	-.001	5	0	3	0	4
237		5	max	203.562	2	.481	4	.746	5	0	1	0	4	0	15
238			min	-301.641	3	.127	15	-.116	1	-.001	5	0	3	0	4
239		6	max	203.697	2	.423	4	.622	5	0	1	0	4	0	15
240			min	-301.54	3	.114	15	-.116	1	-.001	5	0	3	0	4
241		7	max	203.832	2	.366	4	.499	5	0	1	0	4	0	15
242			min	-301.439	3	.1	15	-.116	1	-.001	5	0	3	0	4
243		8	max	203.967	2	.308	4	.376	5	0	1	0	4	0	15
244			min	-301.338	3	.087	15	-.116	1	-.001	5	0	3	0	4
245		9	max	204.102	2	.251	4	.253	5	0	1	.001	4	0	15
246			min	-301.236	3	.07	12	-.116	1	-.001	5	0	3	0	4
247		10	max	204.236	2	.193	4	.13	5	0	1	.001	4	0	15
248			min	-301.135	3	.048	12	-.116	1	-.001	5	0	3	0	4
249		11	max	204.371	2	.136	4	.007	5	0	1	.001	4	0	15
250			min	-301.034	3	.025	12	-.116	1	-.001	5	0	3	0	4
251		12	max	204.506	2	.078	4	.006	3	0	1	.001	5	0	15
252			min	-300.933	3	.002	3	-.135	4	-.001	5	0	3	0	4
253		13	max	204.641	2	.028	5	.006	3	0	1	.001	5	0	15
254			min	-300.832	3	-.032	3	-.258	4	-.001	5	0	3	0	4
255		14	max	204.776	2	.007	5	.006	3	0	1	0	5	0	15
256			min	-300.731	3	-.065	3	-.381	4	-.001	5	0	3	0	4



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	204.911	2	-0.008	15	.006	3	0	1	0	5	0	15
258			min	-300.63	3	-0.099	3	-.505	4	-.001	5	0	3	0	4
259		16	max	205.046	2	-.021	15	.006	3	0	1	0	5	0	15
260			min	-300.528	3	-.153	6	-.628	4	-.001	5	0	3	0	4
261		17	max	205.181	2	-.035	15	.006	3	0	1	0	5	0	12
262			min	-300.427	3	-.21	6	-.751	4	-.001	5	0	3	0	4
263		18	max	205.315	2	-.049	15	.006	3	0	1	0	5	0	12
264			min	-300.326	3	-.268	6	-.874	4	-.001	5	0	3	0	4
265		19	max	205.45	2	-.062	15	.006	3	0	1	0	5	0	12
266			min	-300.225	3	-.325	6	-.997	4	-.001	5	0	1	0	4
267	M11	1	max	229.216	2	1.72	6	.195	1	0	4	0	5	0	2
268			min	-222.4	3	.397	15	-1.256	5	0	10	0	1	0	15
269		2	max	229.146	2	1.544	6	.195	1	0	4	0	3	0	2
270			min	-222.452	3	.356	15	-1.123	5	0	10	0	1	0	3
271		3	max	229.076	2	1.368	6	.195	1	0	4	0	3	0	2
272			min	-222.505	3	.314	15	-.989	5	0	10	0	1	0	3
273		4	max	229.006	2	1.191	6	.195	1	0	4	0	3	0	15
274			min	-222.557	3	.273	15	-.855	5	0	10	0	1	0	4
275		5	max	228.936	2	1.015	6	.195	1	0	4	0	3	0	15
276			min	-222.61	3	.232	15	-.722	5	0	10	0	1	0	4
277		6	max	228.866	2	.839	6	.195	1	0	4	0	3	0	15
278			min	-222.662	3	.19	15	-.588	5	0	10	0	4	0	4
279		7	max	228.796	2	.662	6	.195	1	0	4	0	3	0	15
280			min	-222.715	3	.149	15	-.454	5	0	10	0	4	-.001	4
281		8	max	228.726	2	.486	6	.195	1	0	4	0	3	0	15
282			min	-222.767	3	.107	15	-.321	5	0	10	0	4	-.001	4
283		9	max	228.656	2	.309	6	.195	1	0	4	0	3	0	15
284			min	-222.82	3	.066	15	-.187	5	0	10	0	4	-.001	4
285		10	max	228.586	2	.143	2	.195	1	0	4	0	3	0	15
286			min	-222.872	3	.024	15	-.062	3	0	10	0	4	-.001	4
287		11	max	228.516	2	.006	2	.195	1	0	4	0	3	0	15
288			min	-222.925	3	-.062	3	-.062	3	0	10	0	4	-.001	4
289		12	max	228.446	2	-.059	15	.262	4	0	4	0	3	0	15
290			min	-222.977	3	-.22	4	-.062	3	0	10	0	4	-.001	4
291		13	max	228.376	2	-.1	15	.396	4	0	4	0	3	0	15
292			min	-223.03	3	-.397	4	-.062	3	0	10	0	4	-.001	4
293		14	max	228.306	2	-.142	15	.529	4	0	4	0	3	0	15
294			min	-223.082	3	-.573	4	-.062	3	0	10	0	4	-.001	4
295		15	max	228.236	2	-.183	15	.663	4	0	4	0	3	0	15
296			min	-223.135	3	-.749	4	-.062	3	0	10	0	4	0	4
297		16	max	228.166	2	-.225	15	.797	4	0	4	0	3	0	15
298			min	-223.187	3	-.926	4	-.062	3	0	10	0	5	0	4
299		17	max	228.096	2	-.266	15	.93	4	0	4	0	3	0	15
300			min	-223.24	3	-1.102	4	-.062	3	0	10	0	10	0	4
301		18	max	228.026	2	-.307	15	1.064	4	0	4	0	3	0	15
302			min	-223.292	3	-1.279	4	-.062	3	0	10	0	10	0	4
303		19	max	227.956	2	-.349	15	1.198	4	0	4	0	4	0	1
304			min	-223.345	3	-1.455	4	-.062	3	0	10	0	10	0	1
305	M12	1	max	268.617	1	0	1	1.191	1	0	1	0	4	0	1
306			min	1.975	15	0	1	-16.577	5	0	1	0	3	0	1
307		2	max	268.682	1	0	1	1.191	1	0	1	0	1	0	1
308			min	1.994	15	0	1	-16.633	5	0	1	-.001	5	0	1
309		3	max	268.747	1	0	1	1.191	1	0	1	0	1	0	1
310			min	2.014	15	0	1	-16.689	5	0	1	-.003	5	0	1
311		4	max	268.811	1	0	1	1.191	1	0	1	0	1	0	1
312			min	2.033	15	0	1	-16.745	5	0	1	-.004	5	0	1
313		5	max	268.876	1	0	1	1.191	1	0	1	0	1	0	1









Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	39.886	4	34.304	2	10.36	4	0	3	0	15	.292	3
486			min	1.048	10	-56.047	3	-2.013	2	0	2	-.041	1	-.189	2
487		16	max	34.976	4	44.899	3	24.168	1	0	3	.005	5	.294	3
488			min	1.048	10	-30.072	2	.979	10	0	2	-.035	1	-.19	2
489		17	max	30.065	4	145.845	3	45.887	1	0	3	.01	5	.247	3
490			min	1.048	10	-94.447	2	3.688	10	0	2	-.017	1	-.159	2
491		18	max	25.654	1	246.791	3	67.605	1	0	3	.02	4	.149	3
492			min	1.048	10	-158.823	2	6.396	10	0	2	-.002	10	-.095	2
493		19	max	25.654	1	347.737	3	89.324	1	0	3	.05	1	0	2
494			min	1.048	10	-223.198	2	7.485	12	0	2	.002	10	0	3
495	M16	1	max	33.595	5	341.306	2	4.745	5	0	3	.05	1	0	2
496			min	-26.433	1	-172.752	3	-89.134	1	0	2	-.025	5	0	3
497		2	max	28.685	5	242.602	2	6.262	5	0	3	.011	1	.074	3
498			min	-26.433	1	-123.298	3	-67.415	1	0	2	-.022	5	-.146	2
499		3	max	23.774	5	143.899	2	7.779	5	0	3	0	3	.123	3
500			min	-26.433	1	-73.844	3	-45.697	1	0	2	-.023	4	-.243	2
501		4	max	18.863	5	45.195	2	9.295	5	0	3	-.002	12	.148	3
502			min	-26.433	1	-24.39	3	-23.978	1	0	2	-.035	1	-.29	2
503		5	max	13.952	5	25.063	3	10.812	5	0	3	-.003	12	.148	3
504			min	-26.433	1	-53.508	2	-3.568	3	0	2	-.042	1	-.288	2
505		6	max	9.041	5	74.517	3	19.459	1	0	3	-.003	15	.123	3
506			min	-26.433	1	-152.212	2	-2.141	3	0	2	-.037	1	-.236	2
507		7	max	4.13	5	123.971	3	41.178	1	0	3	.003	5	.073	3
508			min	-26.433	1	-250.915	2	-.714	3	0	2	-.022	1	-.136	2
509		8	max	2.023	3	173.425	3	62.897	1	0	3	.01	4	.015	2
510			min	-26.433	1	-349.619	2	.696	12	0	2	-.007	3	-.001	3
511		9	max	2.023	3	222.879	3	84.615	1	0	3	.041	1	.214	2
512			min	-26.433	1	-448.322	2	1.647	12	0	2	-.006	3	-.1	3
513		10	max	19.507	5	-7.752	15	106.334	1	0	14	.089	1	.463	2
514			min	-26.433	1	-547.026	2	-4.788	3	0	2	-.005	3	-.224	3
515		11	max	14.596	5	448.322	2	2.523	5	0	2	.041	1	.214	2
516			min	-26.369	1	-222.879	3	-84.413	1	0	3	-.01	5	-.1	3
517		12	max	9.685	5	349.619	2	4.04	5	0	2	.006	2	.015	2
518			min	-26.369	1	-173.425	3	-62.694	1	0	3	-.008	5	-.001	3
519		13	max	4.774	5	250.915	2	5.556	5	0	2	0	10	.073	3
520			min	-26.369	1	-123.971	3	-40.975	1	0	3	-.022	1	-.136	2
521		14	max	-.044	15	152.212	2	7.073	5	0	2	-.001	12	.123	3
522			min	-26.369	1	-74.517	3	-19.257	1	0	3	-.037	1	-.236	2
523		15	max	-1.079	10	53.508	2	9.64	4	0	2	.001	5	.148	3
524			min	-26.369	1	-25.063	3	-1.975	2	0	3	-.041	1	-.288	2
525		16	max	-1.079	10	24.391	3	24.181	1	0	2	.006	5	.148	3
526			min	-26.369	1	-45.195	2	1	10	0	3	-.035	1	-.29	2
527		17	max	-1.079	10	73.844	3	45.899	1	0	2	.011	5	.123	3
528			min	-26.369	1	-143.899	2	3.331	12	0	3	-.017	1	-.243	2
529		18	max	-1.079	10	123.298	3	67.618	1	0	2	.021	4	.074	3
530			min	-26.369	1	-242.602	2	4.283	12	0	3	-.002	10	-.146	2
531		19	max	-1.079	10	172.752	3	89.337	1	0	2	.05	1	0	2
532			min	-31.228	4	-341.306	2	5.234	12	0	3	.002	10	0	3
533	M15	1	max	0	1	.939	3	.114	3	0	1	0	1	0	1
534			min	-111.709	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.834	3	.114	3	0	1	0	1	0	1
536			min	-111.785	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.73	3	.114	3	0	1	0	1	0	1
538			min	-111.86	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.626	3	.114	3	0	1	0	1	0	1
540			min	-111.936	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.522	3	.114	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-112.011	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.417	3	.114	3	0	1	0	1	0	1
544		min	-112.087	3	0	1	0	1	0	3	0	3	-.001	3
545	7	max	0	1	.313	3	.114	3	0	1	0	3	0	1
546		min	-112.162	3	0	1	0	1	0	3	0	1	-.001	3
547	8	max	0	1	.209	3	.114	3	0	1	0	3	0	1
548		min	-112.238	3	0	1	0	1	0	3	0	1	-.001	3
549	9	max	0	1	.104	3	.114	3	0	1	0	3	0	1
550		min	-112.314	3	0	1	0	1	0	3	0	1	-.001	3
551	10	max	0	1	0	1	.114	3	0	1	0	3	0	1
552		min	-112.389	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.114	3	0	1	0	3	0	1
554		min	-112.465	3	-.104	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.114	3	0	1	0	3	0	1
556		min	-112.54	3	-.209	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.114	3	0	1	0	3	0	1
558		min	-112.616	3	-.313	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.114	3	0	1	0	3	0	1
560		min	-112.691	3	-.417	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.114	3	0	1	0	3	0	1
562		min	-112.767	3	-.522	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.114	3	0	1	0	3	0	1
564		min	-112.842	3	-.626	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.114	3	0	1	0	3	0	1
566		min	-112.918	3	-.73	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.114	3	0	1	0	3	0	1
568		min	-112.993	3	-.834	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.114	3	0	1	0	3	0	1
570		min	-113.069	3	-.939	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.369	.35	4	0	3	0	3	0	1
572		min	-188.402	4	0	2	-.047	3	0	1	0	4	0	1
573	2	max	0	2	2.106	4	.315	4	0	3	0	3	0	2
574		min	-188.39	4	0	2	-.047	3	0	1	0	4	0	4
575	3	max	0	2	1.842	4	.279	4	0	3	0	3	0	2
576		min	-188.377	4	0	2	-.047	3	0	1	0	4	-.001	4
577	4	max	0	2	1.579	4	.243	4	0	3	0	3	0	2
578		min	-188.365	4	0	2	-.047	3	0	1	0	1	-.002	4
579	5	max	0	2	1.316	4	.207	4	0	3	0	3	0	2
580		min	-188.353	4	0	2	-.047	3	0	1	0	1	-.002	4
581	6	max	0	2	1.053	4	.171	4	0	3	0	3	0	2
582		min	-188.341	4	0	2	-.047	3	0	1	0	1	-.003	4
583	7	max	0	2	.79	4	.136	4	0	3	0	5	0	2
584		min	-188.328	4	0	2	-.047	3	0	1	0	1	-.003	4
585	8	max	0	2	.526	4	.1	4	0	3	0	5	0	2
586		min	-188.316	4	0	2	-.047	3	0	1	0	1	-.003	4
587	9	max	0	2	.263	4	.064	4	0	3	0	5	0	2
588		min	-188.304	4	0	2	-.047	3	0	1	0	1	-.003	4
589	10	max	0	2	0	1	.032	1	0	3	0	5	0	2
590		min	-188.292	4	0	1	-.047	3	0	1	0	1	-.003	4
591	11	max	0	2	0	2	.032	1	0	3	0	5	0	2
592		min	-188.28	4	-.263	4	-.047	3	0	1	0	1	-.003	4
593	12	max	0	2	0	2	.032	1	0	3	0	5	0	2
594		min	-188.267	4	-.526	4	-.047	3	0	1	0	1	-.003	4
595	13	max	0	2	0	2	.032	1	0	3	0	5	0	2
596		min	-188.255	4	-.79	4	-.082	5	0	1	0	3	-.003	4
597	14	max	.078	11	0	2	.032	1	0	3	0	5	0	2
598		min	-188.243	4	-1.053	4	-.118	5	0	1	0	3	-.003	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.162	11	0	2	.032	1	0	3	0	5	0	2
600		min	-188.231	4	-1.316	4	-.153	5	0	1	0	3	-.002	4
601	16	max	.246	11	0	2	.032	1	0	3	0	1	0	2
602		min	-188.218	4	-1.579	4	-.189	5	0	1	0	3	-.002	4
603	17	max	.33	11	0	2	.032	1	0	3	0	1	0	2
604		min	-188.206	4	-1.842	4	-.225	5	0	1	0	3	-.001	4
605	18	max	.413	11	0	2	.032	1	0	3	0	1	0	2
606		min	-188.194	4	-2.106	4	-.261	5	0	1	0	5	0	4
607	19	max	.497	11	0	2	.032	1	0	3	0	1	0	1
608		min	-188.201	5	-2.369	4	-.296	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.011	2	.004	1	1.035e-3	5	NC	3	NC	2
2			min	-.004	3	-.011	3	-.011	5	-4.193e-4	1	3918.474	2	9529.987	1
3		2	max	.002	2	.01	2	.004	1	1.057e-3	5	NC	3	NC	1
4			min	-.004	3	-.011	3	-.011	5	-4.004e-4	1	4285.139	2	NC	1
5		3	max	.002	2	.009	2	.004	1	1.078e-3	5	NC	3	NC	1
6			min	-.003	3	-.01	3	-.011	5	-3.815e-4	1	4722.761	2	NC	1
7		4	max	.002	2	.008	2	.004	1	1.1e-3	5	NC	1	NC	1
8			min	-.003	3	-.01	3	-.011	5	-3.625e-4	1	5248.652	2	NC	1
9		5	max	.002	2	.007	2	.003	1	1.121e-3	5	NC	1	NC	1
10			min	-.003	3	-.009	3	-.01	5	-3.436e-4	1	5885.93	2	NC	1
11		6	max	.002	2	.006	2	.003	1	1.143e-3	5	NC	1	NC	1
12			min	-.003	3	-.009	3	-.01	5	-3.247e-4	1	6665.97	2	NC	1
13		7	max	.001	2	.006	2	.003	1	1.164e-3	5	NC	1	NC	1
14			min	-.003	3	-.008	3	-.01	5	-3.057e-4	1	7632.156	2	NC	1
15		8	max	.001	2	.005	2	.002	1	1.186e-3	5	NC	1	NC	1
16			min	-.002	3	-.008	3	-.009	5	-2.868e-4	1	8845.8	2	NC	1
17		9	max	.001	2	.004	2	.002	1	1.207e-3	5	NC	1	NC	1
18			min	-.002	3	-.007	3	-.009	5	-2.678e-4	1	NC	1	NC	1
19		10	max	.001	2	.003	2	.002	1	1.228e-3	5	NC	1	NC	1
20		min	-.002	3	-.007	3	-.008	5	-2.489e-4	1	NC	1	NC	1	
21	11	max	0	2	.003	2	.001	1	1.25e-3	5	NC	1	NC	1	
22		min	-.002	3	-.006	3	-.008	5	-2.3e-4	1	NC	1	NC	1	
23	12	max	0	2	.002	2	.001	1	1.271e-3	5	NC	1	NC	1	
24		min	-.002	3	-.005	3	-.007	5	-2.11e-4	1	NC	1	NC	1	
25	13	max	0	2	.002	2	0	1	1.293e-3	5	NC	1	NC	1	
26		min	-.001	3	-.005	3	-.006	5	-1.921e-4	1	NC	1	NC	1	
27	14	max	0	2	.001	2	0	1	1.314e-3	5	NC	1	NC	1	
28		min	-.001	3	-.004	3	-.005	5	-1.732e-4	1	NC	1	NC	1	
29	15	max	0	2	0	2	0	1	1.336e-3	5	NC	1	NC	1	
30		min	0	3	-.003	3	-.004	5	-1.542e-4	1	NC	1	NC	1	
31	16	max	0	2	0	2	0	1	1.357e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.003	5	-1.353e-4	1	NC	1	NC	1	
33	17	max	0	2	0	2	0	1	1.379e-3	5	NC	1	NC	1	
34		min	0	3	-.002	3	-.002	5	-1.164e-4	1	NC	1	NC	1	
35	18	max	0	2	0	2	0	1	1.4e-3	5	NC	1	NC	1	
36		min	0	3	0	3	-.001	5	-9.743e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.422e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-7.849e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	3.764e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-6.798e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	4.741e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-6.86e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	5.719e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.923e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.01	5	6.697e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-6.986e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	7.674e-5	1	NC	1	NC	1
48			min	0	2	-.004	3	0	1	-7.049e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.017	4	8.652e-5	1	NC	1	NC	1
50			min	0	2	-.005	3	0	9	-7.111e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.02	4	9.63e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-7.174e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.024	4	1.061e-4	1	NC	1	NC	1
54			min	-.001	2	-.006	3	0	9	-7.237e-4	5	NC	1	NC	1
55		9	max	.001	3	.001	2	.027	4	1.159e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	9	-7.299e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.03	4	1.256e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-7.362e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.033	4	1.354e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-7.425e-4	5	NC	1	NC	1
61		12	max	.002	3	.003	2	.036	4	1.452e-4	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	10	-7.487e-4	5	NC	1	NC	1
63		13	max	.002	3	.004	2	.039	4	1.55e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	10	-7.55e-4	5	NC	1	NC	1
65		14	max	.002	3	.005	2	.042	4	1.647e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	10	-7.613e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.044	4	1.745e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	10	-7.675e-4	5	8441.435	2	NC	1
69		16	max	.002	3	.006	2	.047	4	1.843e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	10	-7.738e-4	5	7173.344	2	NC	1
71		17	max	.002	3	.007	2	.049	4	1.941e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-7.801e-4	5	6188.35	2	NC	1
73		18	max	.002	3	.009	2	.052	4	2.038e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-7.863e-4	5	5414.852	2	NC	1
75		19	max	.003	3	.01	2	.054	4	2.136e-4	1	NC	3	NC	1
76			min	-.003	2	-.009	3	0	10	-7.926e-4	5	4802.213	2	NC	1
77	M4	1	max	.001	1	.013	2	0	10	4.34e-3	5	NC	1	NC	2
78			min	0	15	-.011	3	-.057	4	-3.195e-4	1	NC	1	336.573	4
79		2	max	.001	1	.012	2	0	10	4.34e-3	5	NC	1	NC	2
80			min	0	15	-.011	3	-.053	4	-3.195e-4	1	NC	1	366.877	4
81		3	max	.001	1	.011	2	0	10	4.34e-3	5	NC	1	NC	1
82			min	0	15	-.01	3	-.048	4	-3.195e-4	1	NC	1	402.941	4
83		4	max	.001	1	.01	2	0	10	4.34e-3	5	NC	1	NC	1
84			min	0	15	-.009	3	-.043	4	-3.195e-4	1	NC	1	446.283	4
85		5	max	0	1	.01	2	0	10	4.34e-3	5	NC	1	NC	1
86			min	0	15	-.009	3	-.039	4	-3.195e-4	1	NC	1	498.969	4
87		6	max	0	1	.009	2	0	10	4.34e-3	5	NC	1	NC	1
88			min	0	15	-.008	3	-.034	4	-3.195e-4	1	NC	1	563.873	4
89		7	max	0	1	.008	2	0	10	4.34e-3	5	NC	1	NC	1
90			min	0	15	-.007	3	-.03	4	-3.195e-4	1	NC	1	645.089	4
91		8	max	0	1	.008	2	0	10	4.34e-3	5	NC	1	NC	1
92			min	0	15	-.007	3	-.026	4	-3.195e-4	1	NC	1	748.608	4
93		9	max	0	1	.007	2	0	10	4.34e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.022	4	-3.195e-4	1	NC	1	883.496	4
95		10	max	0	1	.006	2	0	10	4.34e-3	5	NC	1	NC	1
96			min	0	15	-.006	3	-.018	4	-3.195e-4	1	NC	1	1063.998	4
97		11	max	0	1	.006	2	0	10	4.34e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.015	4	-3.195e-4	1	NC	1	1313.579	4
99		12	max	0	1	.005	2	0	10	4.34e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.012	4	-3.195e-4	1	NC	1	1673.165	4
101		max	0	1	.004	2	0	10	4.34e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.009	4	-3.195e-4	1	NC	1	2219.522	4
103		max	0	1	.003	2	0	10	4.34e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.006	4	-3.195e-4	1	NC	1	3110.984	4
105		max	0	1	.003	2	0	10	4.34e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.004	4	-3.195e-4	1	NC	1	4719.516	4
107		max	0	1	.002	2	0	10	4.34e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.002	4	-3.195e-4	1	NC	1	8102.488	4
109		max	0	1	.001	2	0	10	4.34e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.001	4	-3.195e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	4.34e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-3.195e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	4.34e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-3.195e-4	1	NC	1	NC	1
115	M6	max	.007	2	.036	2	.001	9	1.107e-3	4	NC	3	NC	1
116		min	-.012	3	-.034	3	-.011	5	-2.057e-7	1	1194.202	2	6282.727	3
117		max	.006	2	.033	2	.001	9	1.128e-3	4	NC	3	NC	1
118		min	-.011	3	-.033	3	-.011	5	-2.185e-6	1	1279.162	2	6640.973	3
119		max	.006	2	.031	2	.001	9	1.15e-3	4	NC	3	NC	1
120		min	-.011	3	-.031	3	-.011	5	-4.165e-6	1	1376.682	2	7070.571	3
121		max	.006	2	.029	2	.001	9	1.171e-3	4	NC	3	NC	1
122		min	-.01	3	-.029	3	-.011	5	-6.145e-6	1	1489.273	2	7586.143	3
123		max	.005	2	.026	2	.001	9	1.192e-3	4	NC	3	NC	1
124		min	-.009	3	-.027	3	-.011	5	-8.124e-6	1	1620.167	2	8207.203	3
125		max	.005	2	.024	2	0	9	1.214e-3	4	NC	3	NC	1
126		min	-.009	3	-.025	3	-.01	5	-1.01e-5	1	1773.597	2	8960.102	3
127		max	.004	2	.022	2	0	9	1.235e-3	4	NC	3	NC	1
128		min	-.008	3	-.023	3	-.01	5	-1.208e-5	1	1955.206	2	9881.01	3
129		max	.004	2	.02	2	0	9	1.257e-3	4	NC	3	NC	1
130		min	-.007	3	-.022	3	-.01	5	-1.406e-5	1	2172.69	2	NC	1
131		max	.004	2	.017	2	0	1	1.278e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.009	5	-1.604e-5	1	2436.821	2	NC	1
133		max	.003	2	.015	2	0	1	1.3e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.009	5	-1.802e-5	1	2763.146	2	NC	1
135		max	.003	2	.013	2	0	1	1.321e-3	4	NC	3	NC	1
136		min	-.005	3	-.016	3	-.008	5	-2.e-5	1	3174.974	2	NC	1
137		max	.003	2	.011	2	0	1	1.343e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.007	5	-2.198e-5	1	3708.909	2	NC	1
139		max	.002	2	.01	2	0	1	1.364e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.006	5	-2.396e-5	1	4425.916	2	NC	1
141		max	.002	2	.008	2	0	1	1.386e-3	4	NC	1	NC	1
142		min	-.003	3	-.01	3	-.005	5	-2.594e-5	1	5435.677	2	NC	1
143		max	.001	2	.006	2	0	1	1.407e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.004	5	-2.792e-5	1	6957.464	2	NC	1
145		max	.001	2	.004	2	0	1	1.429e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.003	5	-2.99e-5	1	9502.76	2	NC	1
147		max	0	2	.003	2	0	1	1.45e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.002	5	-3.188e-5	1	NC	1	NC	1
149		max	0	2	.001	2	0	1	1.472e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-3.386e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.494e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.584e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.707e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-7.141e-4	5	NC	1	NC	1
155		max	0	3	.001	2	.004	5	1.511e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-7.095e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.007	5	1.316e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-7.052e-4	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.011	5	1.12e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-7.009e-4	4	NC	1	NC	1
161		5	max	.002	3	.006	2	.014	5	9.24e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-6.966e-4	4	8129.703	2	NC	1
163		6	max	.002	3	.007	2	.018	5	2.906e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	-6.923e-4	4	6506.65	2	NC	1
165		7	max	.002	3	.009	2	.021	4	5.537e-5	3	NC	1	NC	1
166			min	-.003	2	-.012	3	0	1	-6.881e-4	4	5399.098	2	NC	1
167		8	max	.003	3	.01	2	.025	4	8.168e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-6.838e-4	4	4589.053	2	NC	1
169		9	max	.003	3	.012	2	.028	4	1.08e-4	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-6.795e-4	4	3967.812	2	NC	1
171		10	max	.004	3	.013	2	.031	4	1.343e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	0	1	-6.752e-4	4	3475.07	2	NC	1
173		11	max	.004	3	.015	2	.034	4	1.606e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	0	1	-6.709e-4	4	3074.595	2	NC	1
175		12	max	.005	3	.017	2	.037	4	1.869e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	0	1	-6.667e-4	4	2743.195	2	NC	1
177		13	max	.005	3	.019	2	.04	4	2.132e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	0	1	-6.624e-4	4	2465.238	2	NC	1
179		14	max	.005	3	.021	2	.043	4	2.396e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	0	1	-6.581e-4	4	2229.722	2	NC	1
181		15	max	.006	3	.023	2	.045	4	2.659e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	0	1	-6.538e-4	4	2028.617	2	NC	1
183		16	max	.006	3	.025	2	.048	4	2.922e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	0	1	-6.495e-4	4	1855.884	2	NC	1
185		17	max	.007	3	.027	2	.051	4	3.185e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	0	1	-6.453e-4	4	1706.872	2	NC	1
187		18	max	.007	3	.029	2	.053	4	3.448e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	0	1	-6.41e-4	4	1577.926	2	NC	1
189		19	max	.007	3	.031	2	.056	4	3.711e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	0	9	-6.367e-4	4	1466.13	2	NC	1
191	M8	1	max	.003	1	.041	2	0	1	4.18e-3	4	NC	1	NC	1
192			min	0	15	-.034	3	-.058	4	-2.793e-4	3	NC	1	332.399	4
193		2	max	.003	1	.039	2	0	1	4.18e-3	4	NC	1	NC	1
194			min	0	15	-.032	3	-.053	4	-2.793e-4	3	NC	1	362.327	4
195		3	max	.003	1	.037	2	0	1	4.18e-3	4	NC	1	NC	1
196			min	0	15	-.03	3	-.049	4	-2.793e-4	3	NC	1	397.944	4
197		4	max	.003	1	.034	2	0	1	4.18e-3	4	NC	1	NC	1
198			min	0	15	-.028	3	-.044	4	-2.793e-4	3	NC	1	440.749	4
199		5	max	.003	1	.032	2	0	1	4.18e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.039	4	-2.793e-4	3	NC	1	492.783	4
201		6	max	.002	1	.03	2	0	1	4.18e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.035	4	-2.793e-4	3	NC	1	556.885	4
203		7	max	.002	1	.027	2	0	1	4.18e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.03	4	-2.793e-4	3	NC	1	637.095	4
205		8	max	.002	1	.025	2	0	1	4.18e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.026	4	-2.793e-4	3	NC	1	739.334	4
207		9	max	.002	1	.023	2	0	1	4.18e-3	4	NC	1	NC	1
208			min	0	15	-.019	3	-.022	4	-2.793e-4	3	NC	1	872.554	4
209		10	max	.002	1	.021	2	0	1	4.18e-3	4	NC	1	NC	1
210			min	0	15	-.017	3	-.018	4	-2.793e-4	3	NC	1	1050.824	4
211		11	max	.002	1	.018	2	0	1	4.18e-3	4	NC	1	NC	1
212			min	0	15	-.015	3	-.015	4	-2.793e-4	3	NC	1	1297.32	4
213		12	max	.001	1	.016	2	0	1	4.18e-3	4	NC	1	NC	1



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 35° 140mph 30psf 4.5ft 7-10.rdb Page 36



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	5.046e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.803e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.486e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-7.306e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.012	4	-7.485e-7	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-7.809e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.015	5	-3.642e-6	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-8.312e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.018	5	-4.155e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-8.816e-4	4	NC	1	NC	1
281		8	max	0	3	.001	2	.021	5	-4.668e-6	10	NC	1	NC	1
282			min	-.001	2	-.006	3	-.002	3	-9.319e-4	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.023	5	-5.181e-6	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	3	-9.822e-4	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.026	5	-5.694e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.032e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.029	5	-6.207e-6	10	NC	1	NC	1
288			min	-.001	2	-.008	3	-.003	3	-1.083e-3	4	NC	1	NC	1
289		12	max	.002	3	.003	2	.031	5	-6.72e-6	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	3	-1.133e-3	4	NC	1	NC	1
291		13	max	.002	3	.004	2	.034	5	-7.233e-6	10	NC	1	NC	1
292			min	-.002	2	-.009	3	-.003	3	-1.183e-3	4	NC	1	NC	1
293		14	max	.002	3	.005	2	.037	5	-7.745e-6	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.003	3	-1.234e-3	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.039	5	-8.258e-6	10	NC	1	NC	1
296			min	-.002	2	-.009	3	-.003	1	-1.284e-3	4	8453.801	2	NC	1
297		16	max	.002	3	.006	2	.041	5	-8.771e-6	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	1	-1.334e-3	4	7182.902	2	NC	1
299		17	max	.002	3	.007	2	.044	5	-9.284e-6	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.004	1	-1.385e-3	4	6195.932	2	NC	1
301		18	max	.002	3	.008	2	.046	5	-9.797e-6	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.004	1	-1.435e-3	4	5421.015	2	NC	1
303		19	max	.003	3	.01	2	.049	5	-1.031e-5	10	NC	3	NC	1
304			min	-.003	2	-.009	3	-.005	1	-1.485e-3	4	4807.342	2	NC	1
305	M12	1	max	.001	1	.013	2	.004	1	4.975e-3	4	NC	1	NC	2
306			min	0	15	-.011	3	-.053	5	1.196e-5	10	NC	1	361.429	5
307		2	max	.001	1	.012	2	.003	1	4.975e-3	4	NC	1	NC	2
308			min	0	15	-.011	3	-.049	5	1.196e-5	10	NC	1	393.961	5
309		3	max	.001	1	.011	2	.003	1	4.975e-3	4	NC	1	NC	2
310			min	0	15	-.01	3	-.045	5	1.196e-5	10	NC	1	432.677	5
311		4	max	.001	1	.01	2	.003	1	4.975e-3	4	NC	1	NC	2
312			min	0	15	-.009	3	-.04	5	1.196e-5	10	NC	1	479.204	5
313		5	max	0	1	.01	2	.003	1	4.975e-3	4	NC	1	NC	2
314			min	0	15	-.009	3	-.036	5	1.196e-5	10	NC	1	535.761	5
315		6	max	0	1	.009	2	.002	1	4.975e-3	4	NC	1	NC	2
316			min	0	15	-.008	3	-.032	5	1.196e-5	10	NC	1	605.434	5
317		7	max	0	1	.008	2	.002	1	4.975e-3	4	NC	1	NC	2
318			min	0	15	-.007	3	-.028	5	1.196e-5	10	NC	1	692.613	5
319		8	max	0	1	.008	2	.002	1	4.975e-3	4	NC	1	NC	1
320			min	0	15	-.007	3	-.024	5	1.196e-5	10	NC	1	803.733	5
321		9	max	0	1	.007	2	.001	1	4.975e-3	4	NC	1	NC	1
322			min	0	15	-.006	3	-.02	5	1.196e-5	10	NC	1	948.521	5
323		10	max	0	1	.006	2	.001	1	4.975e-3	4	NC	1	NC	1
324			min	0	15	-.006	3	-.017	5	1.196e-5	10	NC	1	1142.269	5
325		11	max	0	1	.006	2	0	1	4.975e-3	4	NC	1	NC	1
326			min	0	15	-.005	3	-.014	5	1.196e-5	10	NC	1	1410.16	5
327		12	max	0	1	.005	2	0	1	4.975e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.011	5	1.196e-5	10	NC	1	1796.12	5
329		max	0	1	.004	2	0	1	4.975e-3	4	NC	1	NC	1
330		min	0	15	-.004	3	-.008	5	1.196e-5	10	NC	1	2382.538	5
331		max	0	1	.003	2	0	1	4.975e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.006	5	1.196e-5	10	NC	1	3339.346	5
333		max	0	1	.003	2	0	1	4.975e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.004	5	1.196e-5	10	NC	1	5065.756	5
335		max	0	1	.002	2	0	1	4.975e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.002	5	1.196e-5	10	NC	1	8696.564	5
337		max	0	1	.001	2	0	1	4.975e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.001	5	1.196e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	4.975e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	1.196e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	4.975e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.196e-5	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.007	5	7.186e-3	2	NC	1	NC	1
344		min	-.01	2	-.023	2	-.001	9	-1.072e-2	3	NC	1	NC	1
345		max	.01	3	.017	3	.009	5	3.532e-3	2	NC	4	NC	1
346		min	-.01	2	-.014	2	-.003	1	-5.296e-3	3	5208.248	2	NC	1
347		max	.01	3	.007	3	.011	5	3.643e-4	5	NC	4	NC	1
348		min	-.01	2	-.005	2	-.004	1	-2.39e-4	1	2671.152	2	NC	1
349		max	.01	3	.003	2	.014	5	3.723e-4	5	NC	4	NC	1
350		min	-.01	2	-.002	3	-.005	1	-2.05e-4	1	1865.479	2	6563.959	5
351		max	.01	3	.009	2	.017	5	3.803e-4	5	NC	4	NC	1
352		min	-.01	2	-.009	3	-.005	1	-1.711e-4	1	1458.549	3	4662.777	5
353		max	.01	3	.015	2	.02	5	3.883e-4	5	NC	4	NC	1
354		min	-.01	2	-.014	3	-.005	1	-1.371e-4	1	1238.862	3	3561.443	5
355		max	.01	3	.019	2	.023	5	3.963e-4	5	NC	4	NC	1
356		min	-.01	2	-.019	3	-.004	1	-1.031e-4	1	1110.418	3	2853.277	5
357		max	.01	3	.023	2	.027	5	4.043e-4	5	NC	4	NC	1
358		min	-.01	2	-.022	3	-.003	1	-6.915e-5	1	1033.535	2	2365.627	5
359		max	.01	3	.025	2	.03	5	4.122e-4	5	NC	4	NC	1
360		min	-.01	2	-.024	3	-.002	1	-3.664e-5	9	985.176	2	2013.159	5
361		max	.009	3	.026	2	.034	5	4.202e-4	5	NC	4	NC	1
362		min	-.01	2	-.024	3	-.001	9	-1.2e-5	9	965.508	2	1730.818	4
363		max	.009	3	.026	2	.037	4	4.364e-4	4	NC	4	NC	1
364		min	-.01	2	-.023	3	0	9	3.388e-6	10	972.541	2	1515.689	4
365		max	.009	3	.024	2	.041	4	4.532e-4	4	NC	4	NC	1
366		min	-.01	2	-.021	3	0	10	4.643e-6	10	1008.859	2	1349.879	4
367		max	.009	3	.021	2	.045	4	4.699e-4	4	NC	4	NC	1
368		min	-.01	2	-.018	3	0	10	5.899e-6	10	1083.063	2	1219.952	4
369		max	.009	3	.016	2	.048	4	4.866e-4	4	NC	4	NC	1
370		min	-.01	2	-.014	3	0	10	7.154e-6	10	1214.89	2	1116.949	4
371		max	.009	3	.01	2	.052	4	5.034e-4	4	NC	4	NC	1
372		min	-.01	2	-.008	3	0	10	8.41e-6	10	1450.816	2	1034.73	4
373		max	.009	3	.002	2	.055	4	7.306e-4	4	NC	4	NC	1
374		min	-.01	2	-.002	3	0	10	9.315e-6	10	1855.89	3	968.985	4
375		max	.009	3	.006	3	.057	4	5.968e-3	4	NC	4	NC	1
376		min	-.01	2	-.008	2	0	10	-4.447e-5	9	2693.278	3	916.707	4
377		max	.009	3	.015	3	.06	4	5.354e-3	2	NC	1	NC	1
378		min	-.01	2	-.019	2	0	10	-2.852e-3	3	5281.559	3	875.567	4
379		max	.009	3	.023	3	.062	4	1.08e-2	2	NC	1	NC	1
380		min	-.01	2	-.031	2	0	1	-5.83e-3	3	5656.836	2	844.968	4
381	M5	max	.03	3	.086	3	.007	5	1.713e-5	4	NC	1	NC	1
382		min	-.032	2	-.072	2	-.002	9	4.25e-8	11	3674.07	3	NC	1
383		max	.03	3	.052	3	.009	5	1.813e-4	5	NC	4	NC	1
384		min	-.032	2	-.044	2	-.001	9	-2.336e-5	9	1621.013	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.021	3	.011	5	3.428e-4	5	NC	5	NC	1
386		min	-.032	2	-.016	2	-.001	9	-4.642e-5	9	831.05	2	NC	1
387	4	max	.029	3	.008	2	.014	5	3.592e-4	5	NC	5	NC	1
388		min	-.032	2	-.006	3	-.001	9	-4.439e-5	9	580.067	2	NC	1
389	5	max	.029	3	.03	2	.017	5	3.756e-4	5	NC	5	NC	1
390		min	-.032	2	-.028	3	-.001	9	-4.237e-5	9	458.701	2	8870.913	3
391	6	max	.029	3	.048	2	.021	5	3.919e-4	5	NC	5	NC	1
392		min	-.032	2	-.046	3	-.001	9	-4.034e-5	9	389.529	2	8000.338	3
393	7	max	.029	3	.063	2	.024	5	4.083e-4	5	NC	5	NC	1
394		min	-.032	2	-.059	3	-.001	9	-3.832e-5	9	347.101	2	7595.849	3
395	8	max	.029	3	.074	2	.028	5	4.246e-4	5	NC	5	NC	1
396		min	-.032	2	-.069	3	-.001	9	-3.629e-5	9	320.815	2	7500.76	3
397	9	max	.029	3	.081	2	.032	5	4.41e-4	5	NC	5	NC	1
398		min	-.032	2	-.074	3	-.001	9	-3.427e-5	9	305.721	2	7655.493	3
399	10	max	.029	3	.084	2	.036	5	4.574e-4	5	NC	5	NC	1
400		min	-.032	2	-.076	3	-.001	9	-3.225e-5	9	299.559	2	8052.157	3
401	11	max	.029	3	.083	2	.039	5	4.737e-4	5	NC	5	NC	1
402		min	-.032	2	-.074	3	-.001	9	-3.022e-5	9	301.707	2	8723.047	3
403	12	max	.029	3	.077	2	.043	5	4.901e-4	5	NC	5	NC	1
404		min	-.032	2	-.067	3	0	9	-2.82e-5	9	312.971	2	9747.142	3
405	13	max	.029	3	.067	2	.046	5	5.064e-4	5	NC	5	NC	1
406		min	-.032	2	-.057	3	0	9	-2.617e-5	9	336.029	2	NC	1
407	14	max	.028	3	.052	2	.05	4	5.228e-4	5	NC	5	NC	1
408		min	-.032	2	-.044	3	0	9	-2.415e-5	9	377.042	2	NC	1
409	15	max	.028	3	.032	2	.053	4	5.392e-4	5	NC	5	NC	1
410		min	-.032	2	-.026	3	0	9	-2.212e-5	9	450.535	2	NC	1
411	16	max	.028	3	.006	2	.056	4	7.635e-4	5	NC	5	NC	1
412		min	-.032	2	-.006	3	0	9	-2.182e-5	9	595.279	3	NC	1
413	17	max	.028	3	.019	3	.058	4	5.953e-3	4	NC	5	NC	1
414		min	-.032	2	-.025	2	0	1	-6.327e-5	1	863.565	3	NC	1
415	18	max	.028	3	.045	3	.06	4	3.055e-3	4	NC	4	NC	1
416		min	-.032	2	-.061	2	0	1	-3.242e-5	1	1693.439	3	NC	1
417	19	max	.028	3	.073	3	.062	4	4.632e-6	5	NC	3	NC	1
418		min	-.032	2	-.1	2	0	1	-1.428e-6	3	1732.74	2	NC	1
419	M9	1	max	.01	.026	3	.006	5	1.074e-2	3	NC	1	NC	1
420		min	-.01	2	-.023	2	-.002	9	-7.186e-3	2	NC	1	NC	1
421	2	max	.01	3	.016	3	.005	5	5.269e-3	3	NC	4	NC	1
422		min	-.01	2	-.014	2	0	9	-3.532e-3	2	5208.782	2	NC	1
423	3	max	.01	3	.006	3	.006	4	1.368e-4	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	12	-9.755e-5	3	2664.431	3	NC	1
425	4	max	.01	3	.003	2	.007	4	1.071e-4	1	NC	4	NC	1
426		min	-.01	2	-.002	3	-.001	3	-9.916e-5	3	1807.51	3	NC	1
427	5	max	.01	3	.009	2	.008	4	7.736e-5	1	NC	4	NC	1
428		min	-.01	2	-.009	3	-.003	3	-1.008e-4	3	1424.033	3	8736.491	3
429	6	max	.01	3	.015	2	.01	4	4.762e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.004	3	-1.024e-4	3	1215.651	3	7584.921	3
431	7	max	.01	3	.019	2	.013	4	2.041e-5	4	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-1.04e-4	3	1093.148	3	6545.38	4
433	8	max	.01	3	.023	2	.015	4	3.656e-5	5	NC	4	NC	1
434		min	-.01	2	-.022	3	-.005	3	-1.056e-4	3	1021.27	3	4653.276	4
435	9	max	.01	3	.025	2	.019	4	5.636e-5	5	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-1.072e-4	3	984.102	3	3508.869	4
437	10	max	.01	3	.026	2	.023	5	7.616e-5	5	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-1.088e-4	3	965.604	2	2762.42	4
439	11	max	.009	3	.026	2	.027	5	9.596e-5	5	NC	4	NC	1
440		min	-.01	2	-.024	3	-.005	3	-1.104e-4	3	972.621	2	2247.696	4
441	12	max	.009	3	.024	2	.031	5	1.158e-4	5	NC	4	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.01	2	-.022	3	-.005	3	-1.308e-4	1	1008.916	2	1870.813	5
443		13	max	.009	3	.021	2	.035	5	1.356e-4	5	NC	4	NC	1
444			min	-.01	2	-.018	3	-.005	3	-1.606e-4	1	1083.081	2	1587.909	5
445		14	max	.009	3	.016	2	.04	5	1.554e-4	5	NC	4	NC	1
446			min	-.01	2	-.014	3	-.004	1	-1.903e-4	1	1214.832	2	1376.297	5
447		15	max	.009	3	.01	2	.045	5	1.752e-4	5	NC	4	NC	1
448			min	-.01	2	-.009	3	-.005	1	-2.201e-4	1	1450.587	2	1214.154	5
449		16	max	.009	3	.002	2	.049	5	4.213e-4	5	NC	4	NC	1
450			min	-.01	2	-.002	3	-.005	1	-2.424e-4	1	1843.18	3	1087.569	5
451		17	max	.009	3	.006	3	.053	5	6.064e-3	4	NC	4	NC	1
452			min	-.01	2	-.008	2	-.004	1	-8.957e-5	1	2675.462	3	987.22	5
453		18	max	.009	3	.015	3	.058	5	2.991e-3	5	NC	1	NC	1
454			min	-.01	2	-.019	2	-.003	1	-5.354e-3	2	5247.311	3	902.558	4
455		19	max	.009	3	.024	3	.062	4	5.827e-3	3	NC	1	NC	1
456			min	-.01	2	-.031	2	0	9	-1.08e-2	2	5673.149	2	831.268	4
457	M13	1	max	.002	9	.026	3	.01	3	4.002e-3	3	NC	1	NC	1
458			min	-.006	5	-.023	2	-.01	2	-3.419e-3	2	NC	1	NC	1
459		2	max	.002	9	.089	3	.008	3	4.973e-3	3	NC	4	NC	1
460			min	-.006	5	-.064	2	-.007	2	-4.259e-3	2	1733.98	3	NC	1
461		3	max	.002	9	.141	3	.013	1	5.944e-3	3	NC	4	NC	2
462			min	-.006	5	-.1	2	-.006	10	-5.1e-3	2	944.531	3	6025.952	1
463		4	max	.002	9	.176	3	.021	1	6.915e-3	3	NC	5	NC	2
464			min	-.006	5	-.124	2	-.006	10	-5.94e-3	2	723.749	3	4271.749	1
465		5	max	.002	9	.19	3	.023	1	7.886e-3	3	NC	5	NC	2
466			min	-.006	5	-.135	2	-.008	10	-6.781e-3	2	660.352	3	3912.62	1
467		6	max	.002	9	.184	3	.02	9	8.857e-3	3	NC	5	NC	2
468			min	-.006	5	-.132	2	-.01	2	-7.621e-3	2	685.634	3	4516.094	1
469		7	max	.002	9	.162	3	.019	3	9.828e-3	3	NC	5	NC	2
470			min	-.006	5	-.119	2	-.016	2	-8.462e-3	2	800.581	3	7213.187	9
471		8	max	.002	9	.13	3	.023	3	1.08e-2	3	NC	4	NC	1
472			min	-.006	5	-.1	2	-.023	2	-9.302e-3	2	1048.074	3	7988.595	2
473		9	max	.002	9	.1	3	.026	3	1.177e-2	3	NC	4	NC	1
474			min	-.006	5	-.081	2	-.029	2	-1.014e-2	2	1482.075	3	5500.238	2
475		10	max	.002	9	.086	3	.03	3	1.274e-2	3	NC	4	NC	4
476			min	-.007	5	-.072	2	-.032	2	-1.098e-2	2	1834.541	3	4833.182	2
477		11	max	.002	9	.1	3	.032	3	1.177e-2	3	NC	4	NC	1
478			min	-.007	5	-.081	2	-.029	2	-1.014e-2	2	1482.073	3	4873.598	3
479		12	max	.001	9	.13	3	.033	3	1.08e-2	3	NC	4	NC	1
480			min	-.007	5	-.1	2	-.023	2	-9.302e-3	2	1048.072	3	4735.454	3
481		13	max	.001	9	.162	3	.032	3	9.836e-3	3	NC	5	NC	2
482			min	-.007	5	-.119	2	-.016	2	-8.462e-3	2	800.58	3	4945.42	3
483		14	max	.001	9	.185	3	.029	3	8.868e-3	3	NC	5	NC	2
484			min	-.007	5	-.132	2	-.01	2	-7.621e-3	2	685.634	3	4512.772	1
485		15	max	.001	9	.191	3	.026	3	7.899e-3	3	NC	5	NC	2
486			min	-.007	5	-.135	2	-.008	10	-6.781e-3	2	660.351	3	3917.226	1
487		16	max	.001	9	.176	3	.021	3	6.931e-3	3	NC	5	NC	2
488			min	-.007	5	-.124	2	-.006	10	-5.94e-3	2	723.748	3	4284.334	1
489		17	max	.001	9	.142	3	.017	3	5.962e-3	3	NC	4	NC	2
490			min	-.007	5	-.1	2	-.006	10	-5.1e-3	2	944.53	3	6057.316	1
491		18	max	.001	9	.09	3	.013	3	4.994e-3	3	NC	4	NC	1
492			min	-.007	5	-.064	2	-.008	2	-4.259e-3	2	1733.978	3	NC	1
493		19	max	.001	9	.027	3	.01	3	4.026e-3	3	NC	1	NC	1
494			min	-.007	5	-.023	2	-.01	2	-3.419e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.024	3	.009	3	4.483e-3	2	NC	1	NC	1
496			min	-.062	4	-.031	2	-.01	2	-3.402e-3	3	NC	1	NC	1
497		2	max	0	9	.058	3	.013	3	5.592e-3	2	NC	4	NC	1
498			min	-.062	4	-.094	2	-.007	2	-4.195e-3	3	1721.106	2	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.006	4	-.016	1	-.042	3	-4.784e-3	2	3925.008	1	1056.547	3
557	13	max	.001	3	.007	5	.032	4	5.723e-3	3	NC	5	5009.985	9
558		min	-.006	4	-.014	9	-.041	3	-5.214e-3	2	4250.578	1	1046.839	3
559	14	max	.001	3	.007	5	.028	2	6.164e-3	3	NC	5	5192.118	9
560		min	-.007	4	-.013	9	-.037	3	-5.644e-3	2	4793.058	1	1080.156	3
561	15	max	.002	3	.007	5	.022	2	6.606e-3	3	NC	5	7668.561	15
562		min	-.007	4	-.011	9	-.03	3	-6.075e-3	2	5695.135	1	1173.387	3
563	16	max	.002	3	.007	5	.015	1	7.047e-3	3	NC	4	NC	13
564		min	-.008	4	-.009	9	-.019	3	-6.505e-3	2	7298.553	1	1372.253	3
565	17	max	.002	3	.007	5	.006	4	7.488e-3	3	NC	1	NC	4
566		min	-.008	4	-.007	9	-.005	3	-6.935e-3	2	NC	1	1820.09	3
567	18	max	.002	3	.006	5	.014	3	7.93e-3	3	NC	1	NC	4
568		min	-.009	4	-.004	9	-.015	2	-7.365e-3	2	NC	1	3241.861	3
569	19	max	.002	3	.009	2	.037	3	8.371e-3	3	NC	1	NC	1
570		min	-.009	4	-.001	9	-.035	2	-7.795e-3	2	NC	1	NC	1
571	M16A	1	max	0	.002	2	.011	3	2.365e-3	3	NC	1	NC	1
572		min	-.003	4	-.004	4	-.011	2	-2.347e-3	2	NC	1	NC	1
573	2	max	0	2	-.001	10	.003	3	2.278e-3	3	NC	1	NC	1
574		min	-.003	4	-.01	4	-.005	2	-2.243e-3	2	NC	1	9123.984	3
575	3	max	0	2	-.004	10	.004	1	2.191e-3	3	NC	1	NC	4
576		min	-.003	4	-.016	4	-.008	5	-2.138e-3	2	5621.47	4	5170.738	3
577	4	max	0	2	-.006	12	.007	1	2.104e-3	3	NC	12	NC	9
578		min	-.003	4	-.021	4	-.014	5	-2.033e-3	2	3856.657	4	3940.028	3
579	5	max	0	2	-.007	12	.009	1	2.017e-3	3	NC	12	NC	9
580		min	-.003	4	-.026	4	-.021	5	-1.928e-3	2	3009.389	4	3410.058	3
581	6	max	0	2	-.008	12	.011	1	1.929e-3	3	9586.116	12	NC	9
582		min	-.002	4	-.03	4	-.028	5	-1.823e-3	2	2532.719	4	2625.833	5
583	7	max	0	2	-.009	12	.011	1	1.842e-3	3	8501.155	12	NC	9
584		min	-.002	4	-.033	4	-.034	5	-1.719e-3	2	2246.064	4	2100.263	5
585	8	max	0	2	-.01	12	.011	1	1.755e-3	3	7850.016	12	NC	9
586		min	-.002	4	-.036	4	-.04	5	-1.614e-3	2	2074.029	4	1800.245	5
587	9	max	0	2	-.01	12	.011	1	1.668e-3	3	7499.534	12	NC	9
588		min	-.002	4	-.037	4	-.043	5	-1.509e-3	2	1981.429	4	1631.378	5
589	10	max	0	2	-.01	12	.01	1	1.581e-3	3	7388.658	12	NC	9
590		min	-.002	4	-.037	4	-.045	5	-1.404e-3	2	1952.135	4	1551.44	5
591	11	max	0	2	-.01	12	.008	1	1.494e-3	3	7499.534	12	NC	9
592		min	-.002	4	-.037	4	-.045	5	-1.299e-3	2	1981.429	4	1543.21	5
593	12	max	0	2	-.009	12	.007	1	1.407e-3	3	7850.016	12	NC	9
594		min	-.001	4	-.035	4	-.044	5	-1.195e-3	2	2074.029	4	1605.406	5
595	13	max	0	2	-.009	12	.005	1	1.32e-3	3	8501.155	12	NC	2
596		min	-.001	4	-.032	4	-.04	5	-1.09e-3	2	2246.064	4	1752.272	5
597	14	max	0	2	-.008	12	.004	1	1.233e-3	3	9586.116	12	NC	1
598		min	0	4	-.028	4	-.035	5	-9.85e-4	2	2532.719	4	2021.632	5
599	15	max	0	2	-.006	12	.002	1	1.145e-3	3	NC	12	NC	1
600		min	0	4	-.024	4	-.028	5	-8.802e-4	2	3009.389	4	2500.687	5
601	16	max	0	2	-.005	12	.001	9	1.058e-3	3	NC	12	NC	1
602		min	0	4	-.019	4	-.021	5	-7.754e-4	2	3856.657	4	3408.89	5
603	17	max	0	2	-.003	12	0	9	9.712e-4	3	NC	1	NC	1
604		min	0	4	-.013	4	-.013	5	-6.706e-4	2	5621.47	4	5440.434	5
605	18	max	0	2	-.002	12	0	3	9.953e-4	4	NC	1	NC	1
606		min	0	4	-.006	4	-.006	5	-5.658e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	1.062e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.61e-4	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.