

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

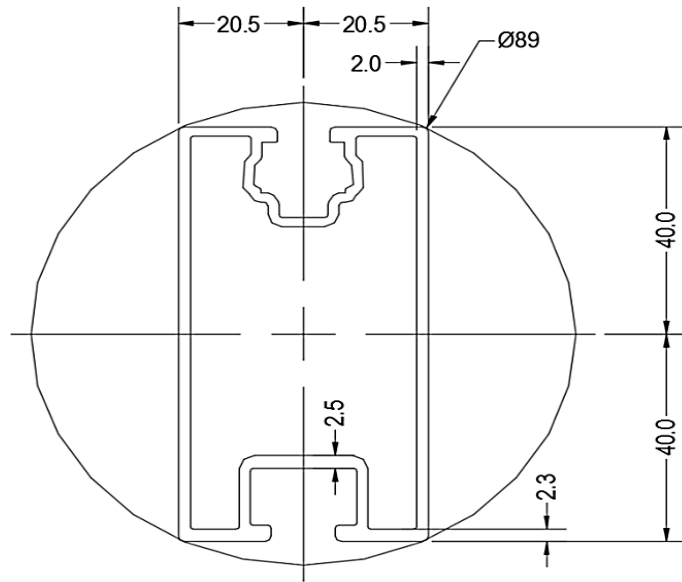
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	75 in
ΦF_{ty} STRONG-AXIS =	29.31 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.789 k-ft
M_z =	0.109 k-ft
$M_{y \text{ allowable}}$ =	1.821 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	56%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.50 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.526 k-ft
M_z =	0.000 k-ft
P_n =	0.176 k
$M_{y \text{ allowable}}$ =	1.448 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.041 k-ft
P_n =	0.249 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	12%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.180 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	5%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

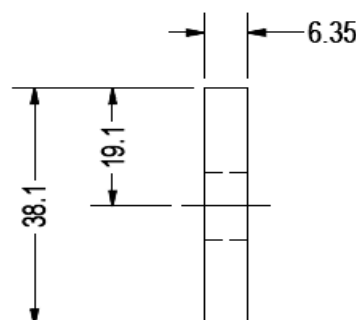
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.118 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<u>14%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.005 k-ft
P_n =	0.215 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>13%</u>



A cross brace kit is required every 15 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

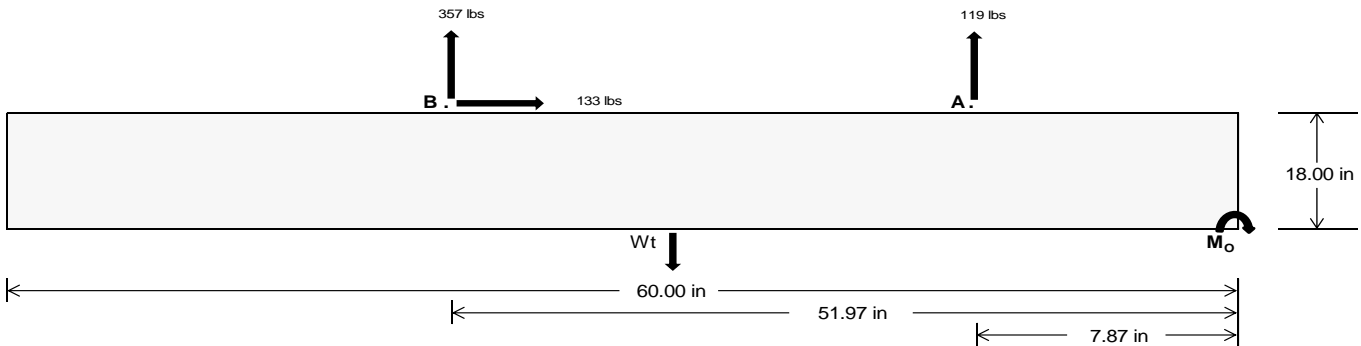
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>521.85</u>	<u>1553.96</u>	k
Compressive Load =	<u>1854.07</u>	<u>1348.97</u>	k
Lateral Load =	<u>33.46</u>	<u>578.22</u>	k
Moment (Weak Axis) =	<u>0.05</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21907.9$ in-lbs
Resisting Force Required = 730.26 lbs
S.F. = 1.67
Weight Required = 1217.11 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 133.38 lbs
Friction = 0.4
Weight Required = 333.45 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 133.38 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	653 lbs	653 lbs	653 lbs	653 lbs	592 lbs	592 lbs	592 lbs	592 lbs	889 lbs	889 lbs	889 lbs	889 lbs	-237 lbs	-237 lbs	-237 lbs	-237 lbs
F_B	476 lbs	476 lbs	476 lbs	476 lbs	431 lbs	431 lbs	431 lbs	431 lbs	646 lbs	646 lbs	646 lbs	646 lbs	-715 lbs	-715 lbs	-715 lbs	-715 lbs
F_V	41 lbs	41 lbs	41 lbs	41 lbs	235 lbs	235 lbs	235 lbs	235 lbs	205 lbs	205 lbs	205 lbs	205 lbs	-267 lbs	-267 lbs	-267 lbs	-267 lbs
P_{total}	3123 lbs	3213 lbs	3304 lbs	3395 lbs	3017 lbs	3107 lbs	3198 lbs	3289 lbs	3529 lbs	3619 lbs	3710 lbs	3801 lbs	244 lbs	298 lbs	353 lbs	407 lbs
M	393 lbs-ft	393 lbs-ft	393 lbs-ft	393 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	762 lbs-ft	762 lbs-ft	762 lbs-ft	762 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	1.93 ft	1.58 ft	1.33 ft	1.16 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	289.2 psf	286.1 psf	283.2 psf	280.6 psf	243.3 psf	242.1 psf	241.1 psf	240.2 psf	285.2 psf	282.3 psf	279.6 psf	277.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	392.1 psf	384.5 psf	377.6 psf	371.2 psf	414.9 psf	406.4 psf	398.5 psf	391.3 psf	484.7 psf	473.1 psf	462.4 psf	452.6 psf	155.2 psf	112.4 psf	100.8 psf	96.9 psf

Maximum Bearing Pressure = 485 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

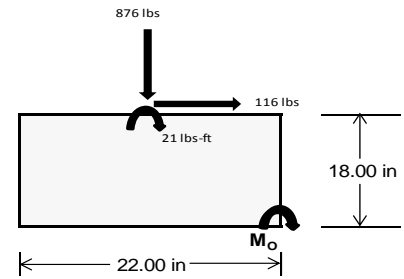
Overturning Check

$M_o = 608.2 \text{ ft-lbs}$
 Resisting Force Required = 663.45 lbs
 S.F. = 1.67
 Weight Required = 1105.75 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	121 lbs	144 lbs	71 lbs	334 lbs	876 lbs	296 lbs	70 lbs	6 lbs	23 lbs
F_v	18 lbs	153 lbs	18 lbs	13 lbs	116 lbs	14 lbs	18 lbs	153 lbs	18 lbs
P_{total}	2589 lbs	2612 lbs	2539 lbs	2684 lbs	3225 lbs	2645 lbs	791 lbs	727 lbs	744 lbs
M	52 lbs-ft	258 lbs-ft	55 lbs-ft	37 lbs-ft	194 lbs-ft	42 lbs-ft	52 lbs-ft	258 lbs-ft	55 lbs-ft
e	0.02 ft	0.10 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.07 ft	0.35 ft	0.07 ft
$L/6$	0.31 ft	1.64 ft	1.79 ft	1.81 ft	1.71 ft	1.80 ft	1.70 ft	1.12 ft	1.69 ft
f_{min}	263.8 sqft	192.9 sqft	257.5 sqft	279.6 sqft	282.4 sqft	273.6 sqft	67.6 sqft	-12.6 sqft	61.7 sqft
f_{max}	301.0 psf	377.0 psf	296.5 psf	305.9 psf	421.3 psf	303.6 psf	105.0 psf	171.3 psf	100.7 psf



Maximum Bearing Pressure = 421 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.505 k
Allowable Uplift =	1.214 k
Utilization =	<u>42%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.113 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.426 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

Diagonal Strut

Maximum Axial Load =	0.180 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.194 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.215 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.086 in
	<u>0.086 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.427$$

$$156.423$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.427$$

$$169.977$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.1$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.3 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.821 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.15 \\ &22.8869 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.5 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.15 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.5 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.448 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

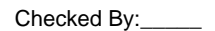
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 15° 130mph 30psf 6.25ft 7-10Pa Page 20



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	346.463	1	.129	6	.507	1	0	10	0	1	0	15
30			min	-368.554	3	.029	15	-.403	5	0	4	0	3	0	6
31		16	max	346.56	1	.091	6	.507	1	0	10	.001	1	0	15
32			min	-368.482	3	.02	15	-.49	5	0	4	0	3	0	6
33		17	max	346.656	1	.058	2	.507	1	0	10	.001	1	0	15
34			min	-368.41	3	.011	15	-.577	5	0	4	0	3	0	6
35		18	max	346.753	1	.03	10	.507	1	0	10	.001	1	0	15
36			min	-368.337	3	-.013	1	-.665	5	0	4	0	3	0	6
37		19	max	346.849	1	.006	10	.507	1	0	10	.001	1	0	15
38			min	-368.265	3	-.042	1	-.752	5	0	4	0	3	0	6
39	M3	1	max	38.695	10	1.81	6	-.023	12	0	5	.001	1	0	6
40			min	-80.584	1	.424	15	-1.426	4	0	1	0	12	0	15
41		2	max	38.639	10	1.632	6	-.023	12	0	5	.001	1	0	6
42			min	-80.651	1	.383	15	-1.292	4	0	1	0	12	0	15
43		3	max	38.583	10	1.454	6	-.023	12	0	5	0	1	0	2
44			min	-80.718	1	.341	15	-1.159	4	0	1	0	12	0	15
45		4	max	38.527	10	1.276	6	-.023	12	0	5	0	1	0	15
46			min	-80.785	1	.299	15	-1.025	4	0	1	0	5	0	1
47		5	max	38.471	10	1.098	6	-.023	12	0	5	0	1	0	15
48			min	-80.853	1	.257	15	-.892	4	0	1	0	5	0	4
49		6	max	38.416	10	.92	6	-.023	12	0	5	0	1	0	15
50			min	-80.92	1	.215	15	-.758	4	0	1	0	5	0	4
51		7	max	38.36	10	.742	6	-.023	12	0	5	0	1	0	15
52			min	-80.987	1	.173	15	-.625	4	0	1	0	5	0	4
53		8	max	38.304	10	.564	6	-.023	12	0	5	0	1	0	15
54			min	-81.054	1	.132	15	-.491	4	0	1	0	5	0	4
55		9	max	38.248	10	.386	6	-.023	12	0	5	0	1	0	15
56			min	-81.121	1	.09	15	-.388	1	0	1	0	5	-.001	4
57		10	max	38.192	10	.208	6	-.023	12	0	5	0	1	0	15
58			min	-81.188	1	.048	15	-.388	1	0	1	0	5	-.001	4
59		11	max	38.136	10	.034	2	-.008	15	0	5	0	1	0	15
60			min	-81.255	1	.006	15	-.388	1	0	1	0	5	-.001	4
61		12	max	38.08	10	-.036	15	.121	5	0	5	0	1	0	15
62			min	-81.322	1	-.148	4	-.388	1	0	1	0	5	-.001	4
63		13	max	38.024	10	-.078	15	.254	5	0	5	0	1	0	15
64			min	-81.389	1	-.326	4	-.388	1	0	1	0	5	-.001	4
65		14	max	37.968	10	-.12	15	.388	5	0	5	0	1	0	15
66			min	-81.456	1	-.505	4	-.388	1	0	1	0	5	-.001	4
67		15	max	37.912	10	-.161	15	.521	5	0	5	0	12	0	15
68			min	-81.523	1	-.683	4	-.388	1	0	1	0	4	0	4
69		16	max	37.856	10	-.203	15	.655	5	0	5	0	12	0	15
70			min	-81.591	1	-.861	4	-.388	1	0	1	0	4	0	4
71		17	max	37.801	10	-.245	15	.788	5	0	5	0	12	0	15
72			min	-81.658	1	-1.039	4	-.388	1	0	1	0	4	0	4
73		18	max	37.745	10	-.287	15	.922	5	0	5	0	12	0	15
74			min	-81.725	1	-1.217	4	-.388	1	0	1	0	1	0	4
75		19	max	37.689	10	-.329	15	1.055	5	0	5	0	5	0	1
76			min	-81.792	1	-1.395	4	-.388	1	0	1	0	1	0	1
77	M4	1	max	475.219	1	0	1	-.078	10	0	1	0	5	0	1
78			min	-116.738	3	0	1	-24.66	4	0	1	0	1	0	1
79		2	max	475.283	1	0	1	-.078	10	0	1	0	12	0	1
80			min	-116.689	3	0	1	-24.716	4	0	1	-.002	4	0	1
81		3	max	475.348	1	0	1	-.078	10	0	1	0	12	0	1
82			min	-116.641	3	0	1	-24.772	4	0	1	-.004	4	0	1
83		4	max	475.413	1	0	1	-.078	10	0	1	0	12	0	1
84			min	-116.592	3	0	1	-24.828	4	0	1	-.007	4	0	1
85		5	max	475.478	1	0	1	-.078	10	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-116.544	3	0	1	-24.885	4	0	1	-.009	4	0	1
87	6	max	475.542	1	0	1	-.078	10	0	1	0	12	0	1
88		min	-116.495	3	0	1	-24.941	4	0	1	-.011	4	0	1
89	7	max	475.607	1	0	1	-.078	10	0	1	0	10	0	1
90		min	-116.447	3	0	1	-24.997	4	0	1	-.013	4	0	1
91	8	max	475.672	1	0	1	-.078	10	0	1	0	10	0	1
92		min	-116.398	3	0	1	-25.053	4	0	1	-.016	4	0	1
93	9	max	475.736	1	0	1	-.078	10	0	1	0	10	0	1
94		min	-116.35	3	0	1	-25.109	4	0	1	-.018	4	0	1
95	10	max	475.801	1	0	1	-.078	10	0	1	0	10	0	1
96		min	-116.301	3	0	1	-25.165	4	0	1	-.02	4	0	1
97	11	max	475.866	1	0	1	-.078	10	0	1	0	10	0	1
98		min	-116.253	3	0	1	-25.221	4	0	1	-.022	4	0	1
99	12	max	475.931	1	0	1	-.078	10	0	1	0	10	0	1
100		min	-116.204	3	0	1	-25.277	4	0	1	-.025	4	0	1
101	13	max	475.995	1	0	1	-.078	10	0	1	0	10	0	1
102		min	-116.156	3	0	1	-25.333	4	0	1	-.027	4	0	1
103	14	max	476.06	1	0	1	-.078	10	0	1	0	10	0	1
104		min	-116.107	3	0	1	-25.389	4	0	1	-.029	4	0	1
105	15	max	476.125	1	0	1	-.078	10	0	1	0	10	0	1
106		min	-116.059	3	0	1	-25.445	4	0	1	-.031	4	0	1
107	16	max	476.189	1	0	1	-.078	10	0	1	0	10	0	1
108		min	-116.01	3	0	1	-25.501	4	0	1	-.034	4	0	1
109	17	max	476.254	1	0	1	-.078	10	0	1	0	10	0	1
110		min	-115.961	3	0	1	-25.557	4	0	1	-.036	4	0	1
111	18	max	476.319	1	0	1	-.078	10	0	1	0	10	0	1
112		min	-115.913	3	0	1	-25.614	4	0	1	-.038	4	0	1
113	19	max	476.383	1	0	1	-.078	10	0	1	0	10	0	1
114		min	-115.864	3	0	1	-25.67	4	0	1	-.041	4	0	1
115	M6	1	max	1116.218	1	.64	.915	4	0	1	0	5	0	1
116			min	-1193.702	3	.149	-.171	3	0	5	0	9	0	1
117	2	max	1116.315	1	.603	6	.827	4	0	1	0	4	0	15
118		min	-1193.63	3	.14	15	-.171	3	0	5	0	9	0	6
119	3	max	1116.411	1	.565	6	.74	4	0	1	0	4	0	15
120		min	-1193.558	3	.131	15	-.171	3	0	5	0	10	0	6
121	4	max	1116.507	1	.527	6	.653	4	0	1	0	4	0	15
122		min	-1193.485	3	.122	15	-.171	3	0	5	0	3	0	6
123	5	max	1116.604	1	.489	6	.565	4	0	1	0	4	0	15
124		min	-1193.413	3	.113	15	-.171	3	0	5	0	3	0	6
125	6	max	1116.7	1	.451	6	.478	4	0	1	0	4	0	15
126		min	-1193.341	3	.105	15	-.171	3	0	5	0	3	0	6
127	7	max	1116.797	1	.413	6	.391	4	0	1	0	4	0	15
128		min	-1193.268	3	.096	15	-.171	3	0	5	0	3	0	6
129	8	max	1116.893	1	.376	6	.303	4	0	1	0	4	0	15
130		min	-1193.196	3	.087	15	-.171	3	0	5	0	3	0	6
131	9	max	1116.989	1	.338	6	.216	4	0	1	0	4	0	15
132		min	-1193.124	3	.078	15	-.171	3	0	5	0	3	0	6
133	10	max	1117.086	1	.3	6	.215	1	0	1	0	4	0	15
134		min	-1193.052	3	.069	15	-.171	3	0	5	0	3	0	6
135	11	max	1117.182	1	.262	6	.215	1	0	1	0	4	0	15
136		min	-1192.979	3	.06	15	-.171	3	0	5	0	3	0	6
137	12	max	1117.278	1	.224	6	.215	1	0	1	0	4	0	15
138		min	-1192.907	3	.051	15	-.171	3	0	5	0	3	0	6
139	13	max	1117.375	1	.192	2	.215	1	0	1	0	4	0	15
140		min	-1192.835	3	.042	15	-.188	5	0	5	0	3	0	6
141	14	max	1117.471	1	.162	2	.215	1	0	1	0	4	0	15
142		min	-1192.763	3	.033	15	-.276	5	0	5	0	3	0	6



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1117.568	1	.133	2	.215	1	0	1	0	4	0	15
144		min	-1192.69	3	.025	15	-.363	5	0	5	0	3	0	6
145	16	max	1117.664	1	.103	2	.215	1	0	1	0	4	0	15
146		min	-1192.618	3	.002	9	-.45	5	0	5	0	3	0	6
147	17	max	1117.76	1	.078	10	.215	1	0	1	0	4	0	15
148		min	-1192.546	3	-.022	9	-.538	5	0	5	0	3	0	6
149	18	max	1117.857	1	.054	10	.215	1	0	1	0	1	0	15
150		min	-1192.473	3	-.05	1	-.625	5	0	5	0	3	0	6
151	19	max	1117.953	1	.029	10	.215	1	0	1	0	1	0	15
152		min	-1192.401	3	-.079	1	-.712	5	0	5	0	3	0	6
153	M7	1	max	179.568	2	1.814	.008	1	0	1	0	4	0	4
154		min	-134.667	9	.43	15	-1.474	4	0	5	0	3	0	15
155	2	max	179.501	2	1.636	4	.008	1	0	1	0	4	0	2
156		min	-134.723	9	.389	15	-1.34	4	0	5	0	3	0	15
157	3	max	179.434	2	1.457	4	.008	1	0	1	0	4	0	2
158		min	-134.779	9	.347	15	-1.207	4	0	5	0	3	0	9
159	4	max	179.367	2	1.279	4	.008	1	0	1	0	1	0	15
160		min	-134.835	9	.305	15	-1.073	4	0	5	0	3	0	1
161	5	max	179.3	2	1.101	4	.008	1	0	1	0	1	0	15
162		min	-134.89	9	.263	15	-.94	4	0	5	0	5	0	6
163	6	max	179.233	2	.923	4	.008	1	0	1	0	1	0	15
164		min	-134.946	9	.221	15	-.806	4	0	5	0	5	0	6
165	7	max	179.166	2	.745	4	.008	1	0	1	0	1	0	15
166		min	-135.002	9	.179	15	-.673	4	0	5	0	5	0	6
167	8	max	179.098	2	.567	4	.008	1	0	1	0	1	0	15
168		min	-135.058	9	.137	15	-.539	4	0	5	0	5	0	6
169	9	max	179.031	2	.389	4	.008	1	0	1	0	1	0	15
170		min	-135.114	9	.096	15	-.405	4	0	5	0	5	-.001	6
171	10	max	178.964	2	.211	4	.008	1	0	1	0	1	0	15
172		min	-135.17	9	.054	15	-.272	4	0	5	0	5	-.001	6
173	11	max	178.897	2	.058	2	.008	1	0	1	0	1	0	15
174		min	-135.226	9	-.011	9	-.138	4	0	5	0	5	-.001	6
175	12	max	178.83	2	-.03	15	.008	1	0	1	0	1	0	15
176		min	-135.282	9	-.145	6	-.008	2	0	5	0	5	-.001	6
177	13	max	178.763	2	-.072	15	.129	5	0	1	0	1	0	15
178		min	-135.338	9	-.323	6	-.008	2	0	5	0	5	-.001	6
179	14	max	178.696	2	-.114	15	.263	5	0	1	0	1	0	15
180		min	-135.394	9	-.501	6	-.008	2	0	5	0	5	-.001	6
181	15	max	178.629	2	-.155	15	.396	5	0	1	0	1	0	15
182		min	-135.45	9	-.679	6	-.008	2	0	5	0	5	0	6
183	16	max	178.562	2	-.197	15	.53	5	0	1	0	1	0	15
184		min	-135.505	9	-.857	6	-.008	2	0	5	0	5	0	6
185	17	max	178.495	2	-.239	15	.664	5	0	1	0	1	0	15
186		min	-135.561	9	-1.035	6	-.008	2	0	5	0	5	0	6
187	18	max	178.428	2	-.281	15	.797	5	0	1	0	1	0	15
188		min	-135.617	9	-1.213	6	-.008	2	0	5	0	5	0	6
189	19	max	178.36	2	-.323	15	.931	5	0	1	0	1	0	1
190		min	-135.673	9	-1.391	6	-.008	2	0	5	0	5	0	1
191	M8	1	max	1425.043	1	0	.55	1	0	1	0	4	0	1
192		min	-402.295	3	0	1	-25.074	4	0	1	0	1	0	1
193	2	max	1425.108	1	0	1	.55	1	0	1	0	1	0	1
194		min	-402.246	3	0	1	-25.13	4	0	1	-.002	4	0	1
195	3	max	1425.172	1	0	1	.55	1	0	1	0	1	0	1
196		min	-402.198	3	0	1	-25.186	4	0	1	-.004	4	0	1
197	4	max	1425.237	1	0	1	.55	1	0	1	0	1	0	1
198		min	-402.149	3	0	1	-25.242	4	0	1	-.007	4	0	1
199	5	max	1425.302	1	0	1	.55	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-402.101	3	0	1	-25.298	4	0	1	-.009	4	0	1
201		6	max	1425.367	1	0	1	.55	1	0	1	0	1	0	1
202			min	-402.052	3	0	1	-25.355	4	0	1	-.011	4	0	1
203		7	max	1425.431	1	0	1	.55	1	0	1	0	1	0	1
204			min	-402.004	3	0	1	-25.411	4	0	1	-.014	4	0	1
205		8	max	1425.496	1	0	1	.55	1	0	1	0	1	0	1
206			min	-401.955	3	0	1	-25.467	4	0	1	-.016	4	0	1
207		9	max	1425.561	1	0	1	.55	1	0	1	0	1	0	1
208			min	-401.907	3	0	1	-25.523	4	0	1	-.018	4	0	1
209		10	max	1425.625	1	0	1	.55	1	0	1	0	1	0	1
210			min	-401.858	3	0	1	-25.579	4	0	1	-.02	4	0	1
211		11	max	1425.69	1	0	1	.55	1	0	1	0	1	0	1
212			min	-401.81	3	0	1	-25.635	4	0	1	-.023	4	0	1
213		12	max	1425.755	1	0	1	.55	1	0	1	0	1	0	1
214			min	-401.761	3	0	1	-25.691	4	0	1	-.025	4	0	1
215		13	max	1425.82	1	0	1	.55	1	0	1	0	1	0	1
216			min	-401.713	3	0	1	-25.747	4	0	1	-.027	4	0	1
217		14	max	1425.884	1	0	1	.55	1	0	1	0	1	0	1
218			min	-401.664	3	0	1	-25.803	4	0	1	-.03	4	0	1
219		15	max	1425.949	1	0	1	.55	1	0	1	0	1	0	1
220			min	-401.615	3	0	1	-25.859	4	0	1	-.032	4	0	1
221		16	max	1426.014	1	0	1	.55	1	0	1	0	1	0	1
222			min	-401.567	3	0	1	-25.915	4	0	1	-.034	4	0	1
223		17	max	1426.078	1	0	1	.55	1	0	1	0	1	0	1
224			min	-401.518	3	0	1	-25.971	4	0	1	-.037	4	0	1
225		18	max	1426.143	1	0	1	.55	1	0	1	0	1	0	1
226			min	-401.47	3	0	1	-26.028	4	0	1	-.039	4	0	1
227		19	max	1426.208	1	0	1	.55	1	0	1	0	1	0	1
228			min	-401.421	3	0	1	-26.084	4	0	1	-.041	4	0	1
229	M10	1	max	347.631	1	.685	4	1.103	5	0	1	0	4	0	1
230			min	-353.9	3	.172	15	-.073	1	-.002	5	0	3	0	1
231		2	max	347.727	1	.648	4	1.015	5	0	1	0	4	0	15
232			min	-353.828	3	.163	15	-.073	1	-.002	5	0	3	0	4
233		3	max	347.824	1	.61	4	.928	5	0	1	0	4	0	15
234			min	-353.755	3	.154	15	-.073	1	-.002	5	0	3	0	4
235		4	max	347.92	1	.572	4	.841	5	0	1	0	4	0	15
236			min	-353.683	3	.145	15	-.073	1	-.002	5	0	3	0	4
237		5	max	348.016	1	.534	4	.753	5	0	1	0	4	0	15
238			min	-353.611	3	.136	15	-.073	1	-.002	5	0	3	0	4
239		6	max	348.113	1	.496	4	.666	5	0	1	0	4	0	15
240			min	-353.539	3	.127	15	-.073	1	-.002	5	0	3	0	4
241		7	max	348.209	1	.459	4	.579	5	0	1	0	4	0	15
242			min	-353.466	3	.118	15	-.073	1	-.002	5	0	3	0	4
243		8	max	348.306	1	.421	4	.491	5	0	1	0	4	0	15
244			min	-353.394	3	.11	15	-.073	1	-.002	5	0	3	0	4
245		9	max	348.402	1	.383	4	.404	5	0	1	0	4	0	15
246			min	-353.322	3	.101	15	-.073	1	-.002	5	0	3	0	4
247		10	max	348.498	1	.345	4	.317	5	0	1	.001	4	0	15
248			min	-353.249	3	.092	15	-.073	1	-.002	5	0	3	0	4
249		11	max	348.595	1	.307	4	.229	5	0	1	.001	4	0	15
250			min	-353.177	3	.083	15	-.073	1	-.002	5	0	3	0	4
251		12	max	348.691	1	.269	4	.142	5	0	1	.001	4	0	15
252			min	-353.105	3	.074	15	-.073	1	-.002	5	0	3	0	4
253		13	max	348.787	1	.232	4	.055	5	0	1	.001	4	0	15
254			min	-353.033	3	.065	15	-.073	1	-.002	5	0	3	0	4
255		14	max	348.884	1	.194	4	-.021	10	0	1	.001	5	0	15
256			min	-352.96	3	.056	15	-.073	1	-.002	5	0	3	0	4





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-116.179	3	0	1	-23.157	5	0	1	-.008	5	0	1
315		6	max	475.507	1	0	1	1.879	1	0	1	0	1	0	1
316			min	-116.131	3	0	1	-23.214	5	0	1	-.01	5	0	1
317		7	max	475.572	1	0	1	1.879	1	0	1	.001	1	0	1
318			min	-116.082	3	0	1	-23.27	5	0	1	-.012	5	0	1
319		8	max	475.637	1	0	1	1.879	1	0	1	.001	1	0	1
320			min	-116.034	3	0	1	-23.326	5	0	1	-.014	5	0	1
321		9	max	475.701	1	0	1	1.879	1	0	1	.001	1	0	1
322			min	-115.985	3	0	1	-23.382	5	0	1	-.017	5	0	1
323		10	max	475.766	1	0	1	1.879	1	0	1	.002	1	0	1
324			min	-115.937	3	0	1	-23.438	5	0	1	-.019	5	0	1
325		11	max	475.831	1	0	1	1.879	1	0	1	.002	1	0	1
326			min	-115.888	3	0	1	-23.494	5	0	1	-.021	5	0	1
327		12	max	475.895	1	0	1	1.879	1	0	1	.002	1	0	1
328			min	-115.839	3	0	1	-23.55	5	0	1	-.023	5	0	1
329		13	max	475.96	1	0	1	1.879	1	0	1	.002	1	0	1
330			min	-115.791	3	0	1	-23.606	5	0	1	-.025	5	0	1
331		14	max	476.025	1	0	1	1.879	1	0	1	.002	1	0	1
332			min	-115.742	3	0	1	-23.662	5	0	1	-.027	5	0	1
333		15	max	476.09	1	0	1	1.879	1	0	1	.002	1	0	1
334			min	-115.694	3	0	1	-23.718	5	0	1	-.029	5	0	1
335		16	max	476.154	1	0	1	1.879	1	0	1	.003	1	0	1
336			min	-115.645	3	0	1	-23.774	5	0	1	-.031	5	0	1
337		17	max	476.219	1	0	1	1.879	1	0	1	.003	1	0	1
338			min	-115.597	3	0	1	-23.83	5	0	1	-.033	5	0	1
339		18	max	476.284	1	0	1	1.879	1	0	1	.003	1	0	1
340			min	-115.548	3	0	1	-23.886	5	0	1	-.036	5	0	1
341		19	max	476.348	1	0	1	1.879	1	0	1	.003	1	0	1
342			min	-115.5	3	0	1	-23.943	5	0	1	-.038	5	0	1
343	M1	1	max	85.254	1	346.255	3	-1.685	12	0	1	.072	1	.015	1
344			min	3.368	12	-346.449	1	-36.516	1	0	3	.004	12	-.013	3
345		2	max	85.326	1	346.052	3	-1.685	12	0	1	.064	1	.09	1
346			min	3.404	12	-346.719	1	-36.516	1	0	3	.004	12	-.088	3
347		3	max	97.925	1	5.615	9	-1.737	12	0	5	.055	1	.164	1
348			min	-7.324	3	-23.522	3	-36.199	1	0	1	.003	12	-.161	3
349		4	max	97.997	1	5.39	9	-1.737	12	0	5	.047	1	.165	1
350			min	-7.27	3	-23.725	3	-36.199	1	0	1	.003	12	-.156	3
351		5	max	98.069	1	5.165	9	-1.737	12	0	5	.039	1	.165	1
352			min	-7.216	3	-23.927	3	-36.199	1	0	1	.002	12	-.151	3
353		6	max	98.141	1	4.941	9	-1.737	12	0	5	.032	1	.166	1
354			min	-7.162	3	-24.129	3	-36.199	1	0	1	.002	12	-.146	3
355		7	max	98.214	1	4.716	9	-1.737	12	0	5	.024	1	.166	1
356			min	-7.107	3	-24.332	3	-36.199	1	0	1	.002	10	-.141	3
357		8	max	98.286	1	4.491	9	-1.737	12	0	5	.016	1	.167	1
358			min	-7.053	3	-24.534	3	-36.199	1	0	1	.001	10	-.135	3
359		9	max	98.358	1	4.266	9	-1.737	12	0	5	.008	1	.167	1
360			min	-6.999	3	-24.736	3	-36.199	1	0	1	0	10	-.13	3
361		10	max	98.431	1	4.042	9	-1.737	12	0	5	.002	4	.168	1
362			min	-6.945	3	-24.939	3	-36.199	1	0	1	0	10	-.125	3
363		11	max	98.503	1	3.817	9	-1.737	12	0	5	0	3	.169	1
364			min	-6.891	3	-25.141	3	-36.199	1	0	1	-.008	1	-.119	3
365		12	max	98.575	1	3.592	9	-1.737	12	0	5	0	12	.17	1
366			min	-6.836	3	-25.343	3	-36.199	1	0	1	-.016	1	-.114	3
367		13	max	98.647	1	3.367	9	-1.737	12	0	5	0	12	.171	1
368			min	-6.782	3	-25.545	3	-36.199	1	0	1	-.023	1	-.108	3
369		14	max	98.72	1	3.143	9	-1.737	12	0	5	-.001	12	.172	1
370			min	-6.728	3	-25.748	3	-36.199	1	0	1	-.031	1	-.103	3







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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	50.862	4	58.869	1	9.656	4	.013	3	0	5	.398	3
486		min	1.685	12	-59.55	3	-.134	10	-.015	1	-.053	1	-.399	1
487	16	max	43.986	4	41.897	3	24.523	1	.013	3	.007	5	.404	3
488		min	1.685	12	-42.368	1	1.555	12	-.015	1	-.043	1	-.405	1
489	17	max	37.11	4	143.343	3	44.765	1	.013	3	.014	5	.34	3
490		min	1.685	12	-143.605	1	2.159	12	-.015	1	-.019	1	-.34	1
491	18	max	36.589	1	244.789	3	65.007	1	.013	3	.026	4	.205	3
492		min	1.685	12	-244.843	1	2.764	12	-.015	1	0	10	-.205	1
493	19	max	36.589	1	346.236	3	85.249	1	.013	3	.072	1	0	1
494		min	1.685	12	-346.08	1	3.368	12	-.015	1	.004	12	0	3
495	M16	1	max	47.759	5	398.635	1	6.598	5	.009	3	.071	1	0
496		min	-37.394	1	-164.687	3	-84.927	1	-.016	1	-.035	5	0	3
497	2	max	40.883	5	281.981	1	7.556	5	.009	3	.019	1	.098	3
498		min	-37.394	1	-116.683	3	-64.685	1	-.016	1	-.03	5	-.236	1
499	3	max	34.007	5	165.327	1	8.514	5	.009	3	0	12	.162	3
500		min	-37.394	1	-68.68	3	-44.443	1	-.016	1	-.028	4	-.392	1
501	4	max	27.131	5	48.674	1	9.473	5	.009	3	-.002	12	.193	3
502		min	-37.394	1	-20.677	3	-24.201	1	-.016	1	-.043	1	-.466	1
503	5	max	20.255	5	27.327	3	10.431	5	.009	3	-.002	12	.191	3
504		min	-37.394	1	-67.98	1	-3.96	1	-.016	1	-.053	1	-.459	1
505	6	max	13.379	5	75.33	3	16.282	1	.009	3	-.002	12	.155	3
506		min	-37.394	1	-184.633	1	.02	3	-.016	1	-.049	1	-.372	1
507	7	max	6.503	5	123.333	3	36.524	1	.009	3	.005	5	.086	3
508		min	-37.394	1	-301.287	1	.663	12	-.016	1	-.03	1	-.203	1
509	8	max	.377	3	171.337	3	56.766	1	.009	3	.014	4	.047	1
510		min	-37.394	1	-417.941	1	1.268	12	-.016	1	-.002	3	-.016	3
511	9	max	.377	3	219.34	3	77.008	1	.009	3	.049	1	.378	1
512		min	-37.394	1	-534.594	1	1.872	12	-.016	1	0	3	-.152	3
513	10	max	28.021	5	-12.898	15	97.25	1	.006	14	.109	1	.789	1
514		min	-37.394	1	-651.248	1	-3.982	3	-.016	1	.003	12	-.321	3
515	11	max	21.145	5	534.594	1	4.474	5	.016	1	.048	1	.378	1
516		min	-37.257	1	-219.34	3	-76.704	1	-.009	3	-.015	5	-.152	3
517	12	max	14.269	5	417.941	1	5.432	5	.016	1	.002	2	.047	1
518		min	-37.257	1	-171.337	3	-56.463	1	-.009	3	-.012	5	-.016	3
519	13	max	7.393	5	301.287	1	6.39	5	.016	1	0	12	.086	3
520		min	-37.257	1	-123.333	3	-36.221	1	-.009	3	-.031	1	-.203	1
521	14	max	.516	5	184.633	1	7.349	5	.016	1	-.001	12	.155	3
522		min	-37.257	1	-75.33	3	-15.979	1	-.009	3	-.049	1	-.372	1
523	15	max	-1.864	12	67.98	1	9.301	4	.016	1	.003	5	.191	3
524		min	-37.257	1	-27.327	3	-.137	10	-.009	3	-.053	1	-.459	1
525	16	max	-1.864	12	20.677	3	24.505	1	.016	1	.009	5	.193	3
526		min	-37.257	1	-48.674	1	.95	12	-.009	3	-.043	1	-.466	1
527	17	max	-1.864	12	68.68	3	44.747	1	.016	1	.015	5	.162	3
528		min	-37.257	1	-165.327	1	1.554	12	-.009	3	-.019	1	-.392	1
529	18	max	-1.864	12	116.683	3	64.989	1	.016	1	.027	4	.098	3
530		min	-37.257	1	-281.981	1	2.159	12	-.009	3	0	10	-.236	1
531	19	max	-1.864	12	164.687	3	85.231	1	.016	1	.072	1	0	1
532		min	-41.392	4	-398.635	1	2.763	12	-.009	3	.003	12	0	5
533	M15	1	max	0	1.416	9	.046	3	0	9	0	9	0	1
534		min	-38.544	3	0	1	-.024	9	0	3	0	3	0	1
535	2	max	0	1	1.259	9	.046	3	0	9	0	9	0	1
536		min	-38.598	3	0	1	-.024	9	0	3	0	3	0	9
537	3	max	0	1	1.101	9	.046	3	0	9	0	9	0	1
538		min	-38.652	3	0	1	-.024	9	0	3	0	3	0	9
539	4	max	0	1	.944	9	.046	3	0	9	0	9	0	1
540		min	-38.706	3	0	1	-.024	9	0	3	0	3	-.001	9
541	5	max	0	1	.787	9	.046	3	0	9	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-38.76	3	0	1	-.024	9	0	3	0	3	-.002	9
543		6	max	0	1	.629	9	.046	3	0	9	0	9	0	1
544			min	-38.814	3	0	1	-.024	9	0	3	0	3	-.002	9
545		7	max	0	1	.472	9	.046	3	0	9	0	3	0	1
546			min	-38.868	3	0	1	-.024	9	0	3	0	9	-.002	9
547		8	max	0	1	.315	9	.046	3	0	9	0	3	0	1
548			min	-38.922	3	0	1	-.024	9	0	3	0	9	-.002	9
549		9	max	0	1	.157	9	.046	3	0	9	0	3	0	1
550			min	-38.976	3	0	1	-.024	9	0	3	0	9	-.002	9
551		10	max	0	1	0	1	.046	3	0	9	0	3	0	1
552			min	-39.03	3	0	1	-.024	9	0	3	0	9	-.002	9
553		11	max	0	1	0	1	.046	3	0	9	0	3	0	1
554			min	-39.084	3	-.157	9	-.024	9	0	3	0	9	-.002	9
555		12	max	0	1	0	1	.046	3	0	9	0	3	0	1
556			min	-39.138	3	-.315	9	-.024	9	0	3	0	9	-.002	9
557		13	max	0	1	0	1	.046	3	0	9	0	3	0	1
558			min	-39.192	3	-.472	9	-.024	9	0	3	0	9	-.002	9
559		14	max	0	1	0	1	.046	3	0	9	0	3	0	1
560			min	-39.246	3	-.629	9	-.024	9	0	3	0	9	-.002	9
561		15	max	0	1	0	1	.046	3	0	9	0	3	0	1
562			min	-39.3	3	-.787	9	-.024	9	0	3	0	9	-.002	9
563		16	max	0	1	0	1	.046	3	0	9	0	3	0	1
564			min	-39.354	3	-.944	9	-.024	9	0	3	0	9	-.001	9
565		17	max	0	1	0	1	.046	3	0	9	0	3	0	1
566			min	-39.408	3	-1.101	9	-.024	9	0	3	0	9	0	9
567		18	max	0	1	0	1	.046	3	0	9	0	3	0	1
568			min	-39.462	3	-1.259	9	-.024	9	0	3	0	9	0	9
569		19	max	0	1	0	1	.046	3	0	9	0	3	0	1
570			min	-39.516	3	-1.416	9	-.024	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.738	4	.217	4	0	3	0	3	0	1
572			min	-213.898	4	0	10	-.02	3	0	1	0	4	0	1
573		2	max	0	10	2.434	4	.197	4	0	3	0	3	0	10
574			min	-213.968	4	0	10	-.02	3	0	1	0	4	0	4
575		3	max	0	10	2.13	4	.176	4	0	3	0	3	0	10
576			min	-214.038	4	0	10	-.02	3	0	1	0	4	-.002	4
577		4	max	0	10	1.825	4	.156	4	0	3	0	3	0	10
578			min	-214.108	4	0	10	-.02	3	0	1	0	1	-.003	4
579		5	max	0	10	1.521	4	.135	4	0	3	0	3	0	10
580			min	-214.178	4	0	10	-.02	3	0	1	0	1	-.003	4
581		6	max	0	10	1.217	4	.115	4	0	3	0	3	0	10
582			min	-214.248	4	0	10	-.02	3	0	1	0	1	-.004	4
583		7	max	0	10	.913	4	.095	4	0	3	0	5	0	10
584			min	-214.318	4	0	10	-.02	3	0	1	0	1	-.004	4
585		8	max	0	10	.608	4	.074	4	0	3	0	5	0	10
586			min	-214.389	4	0	10	-.02	3	0	1	0	1	-.004	4
587		9	max	0	10	.304	4	.054	4	0	3	0	5	0	10
588			min	-214.459	4	0	10	-.02	3	0	1	0	1	-.005	4
589		10	max	0	10	0	1	.033	4	0	3	0	5	0	10
590			min	-214.529	4	0	1	-.02	3	0	1	0	1	-.005	4
591		11	max	0	10	0	10	.028	1	0	3	0	5	0	10
592			min	-214.599	4	-.304	4	-.02	3	0	1	0	1	-.005	4
593		12	max	0	10	0	10	.028	1	0	3	0	5	0	10
594			min	-214.669	4	-.608	4	-.02	3	0	1	0	1	-.004	4
595		13	max	0	10	0	10	.028	1	0	3	0	5	0	10
596			min	-214.739	4	-.913	4	-.032	5	0	1	0	9	-.004	4
597		14	max	0	10	0	10	.028	1	0	3	0	4	0	10
598			min	-214.81	4	-1.217	4	-.052	5	0	1	0	3	-.004	4



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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	10	0	10	.028	1	0	3	0	4	0	10
600		min	-214.88	4	-1.521	4	-.073	5	0	1	0	3	-.003	4
601	16	max	0	10	0	10	.028	1	0	3	0	4	0	10
602		min	-214.95	4	-1.825	4	-.093	5	0	1	0	3	-.003	4
603	17	max	.02	2	0	10	.028	1	0	3	0	1	0	10
604		min	-215.02	4	-2.13	4	-.114	5	0	1	0	3	-.002	4
605	18	max	.092	2	0	10	.028	1	0	3	0	1	0	10
606		min	-215.09	4	-2.434	4	-.134	5	0	1	0	3	0	4
607	19	max	.164	2	0	10	.028	1	0	3	0	1	0	1
608		min	-215.16	4	-2.738	4	-.154	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.007	1	1.378e-3	5	NC	3	NC	2	
2			min	-.003	3	-.005	3	-.014	5	-5.003e-4	1	4848.01	2	4043.719	1	
3			2	max	.002	1	.006	2	.007	1	1.401e-3	5	NC	3	NC	2
4				min	-.003	3	-.005	3	-.013	5	-4.812e-4	1	5248.787	2	4385.058	1
5			3	max	.002	1	.005	2	.006	1	1.425e-3	5	NC	3	NC	2
6				min	-.002	3	-.005	3	-.012	5	-4.622e-4	1	5718.431	2	4786.91	1
7			4	max	.002	1	.005	2	.006	1	1.448e-3	5	NC	3	NC	2
8				min	-.002	3	-.004	3	-.012	5	-4.431e-4	1	6272.479	2	5264.088	1
9			5	max	.002	1	.004	2	.005	1	1.472e-3	5	NC	3	NC	2
10				min	-.002	3	-.004	3	-.011	5	-4.241e-4	1	6931.345	2	5836.381	1
11			6	max	.002	1	.004	2	.005	1	1.495e-3	5	NC	1	NC	2
12				min	-.002	3	-.004	3	-.011	5	-4.05e-4	1	7722.276	2	6530.712	1
13			7	max	.002	1	.003	2	.004	1	1.519e-3	5	NC	1	NC	2
14				min	-.002	3	-.004	3	-.01	5	-3.859e-4	1	8682.33	2	7384.488	1
15			8	max	.002	1	.003	2	.004	1	1.542e-3	5	NC	1	NC	2
16				min	-.002	3	-.004	3	-.009	5	-3.669e-4	1	9863.017	2	8450.955	1
17			9	max	.001	1	.003	2	.003	1	1.566e-3	5	NC	1	NC	2
18				min	-.002	3	-.003	3	-.008	5	-3.478e-4	1	NC	1	9808.073	1
19			10	max	.001	1	.002	2	.003	1	1.589e-3	5	NC	1	NC	1
20				min	-.001	3	-.003	3	-.008	5	-3.287e-4	1	NC	1	NC	1
21		11	max	.001	1	.002	2	.002	1	1.613e-3	5	NC	1	NC	1	
22			min	-.001	3	-.003	3	-.007	5	-3.097e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	1.636e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.006	5	-2.906e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	.001	1	1.66e-3	5	NC	1	NC	1	
26			min	0	3	-.002	3	-.005	5	-2.715e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.001	1	1.683e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.004	5	-2.525e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.706e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.004	5	-2.334e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.73e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.003	5	-2.143e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.753e-3	5	NC	1	NC	1	
34			min	0	3	0	3	-.002	5	-1.953e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.777e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-1.762e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.8e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.571e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	7.134e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-8.196e-4	5	NC	1	NC	1	
41			2	max	0	1	0	2	.004	5	8.983e-5	1	NC	1	NC	1
42				min	0	10	0	3	0	1	-8.236e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	1	0	2	.009	5	1.083e-4	1	NC	1	NC	1
44			min	0	10	-.001	3	0	1	-8.276e-4	5	NC	1	NC	1
45		4	max	0	1	0	2	.013	5	1.268e-4	1	NC	1	NC	1
46			min	0	10	-.002	3	0	1	-8.315e-4	5	NC	1	NC	1
47		5	max	0	1	0	2	.018	4	1.453e-4	1	NC	1	NC	1
48			min	0	10	-.003	3	0	1	-8.355e-4	5	NC	1	NC	1
49		6	max	0	1	0	2	.022	4	1.638e-4	1	NC	1	NC	1
50			min	0	10	-.003	3	0	1	-8.395e-4	5	NC	1	NC	1
51		7	max	0	1	0	2	.026	4	1.823e-4	1	NC	1	NC	1
52			min	0	10	-.004	3	0	1	-8.435e-4	5	NC	1	NC	1
53		8	max	0	1	.001	2	.031	4	2.007e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	-8.474e-4	5	NC	1	NC	1
55		9	max	0	1	.002	2	.035	4	2.192e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	1	-8.514e-4	5	NC	1	NC	1
57		10	max	0	1	.002	2	.039	4	2.377e-4	1	NC	1	NC	1
58			min	0	10	-.006	3	0	10	-8.554e-4	5	NC	1	NC	1
59		11	max	0	1	.002	2	.044	4	2.562e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	10	-8.594e-4	5	NC	1	NC	1
61		12	max	0	1	.003	2	.048	4	2.747e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	10	-8.633e-4	5	NC	1	NC	1
63		13	max	0	1	.004	2	.052	4	2.932e-4	1	NC	1	NC	1
64			min	0	10	-.007	3	0	10	-8.673e-4	5	NC	1	NC	1
65		14	max	0	1	.005	2	.056	4	3.116e-4	1	NC	1	NC	1
66			min	0	10	-.007	3	0	10	-8.713e-4	5	NC	1	NC	1
67		15	max	0	1	.005	2	.06	4	3.301e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	10	-8.753e-4	5	8516.468	2	NC	1
69		16	max	0	1	.006	2	.063	4	3.486e-4	1	NC	3	NC	1
70			min	0	10	-.007	3	0	10	-8.793e-4	5	7270.682	2	NC	1
71		17	max	0	1	.007	2	.067	4	3.671e-4	1	NC	3	NC	1
72			min	0	10	-.007	3	0	10	-8.832e-4	5	6294.299	2	NC	1
73		18	max	0	1	.008	2	.071	4	3.856e-4	1	NC	3	NC	1
74			min	0	10	-.007	3	0	10	-8.872e-4	5	5522.322	2	NC	1
75		19	max	0	1	.009	2	.075	4	4.041e-4	1	NC	3	NC	1
76			min	0	10	-.007	3	0	10	-8.912e-4	5	4907.747	2	NC	1
77	M4	1	max	.002	1	.007	2	0	10	3.219e-3	5	NC	1	NC	2
78			min	0	3	-.005	3	-.079	4	-4.66e-4	1	NC	1	244.566	4
79		2	max	.002	1	.007	2	0	10	3.219e-3	5	NC	1	NC	2
80			min	0	3	-.005	3	-.072	4	-4.66e-4	1	NC	1	266.604	4
81		3	max	.002	1	.006	2	0	10	3.219e-3	5	NC	1	NC	2
82			min	0	3	-.005	3	-.066	4	-4.66e-4	1	NC	1	292.834	4
83		4	max	.002	1	.006	2	0	10	3.219e-3	5	NC	1	NC	1
84			min	0	3	-.004	3	-.06	4	-4.66e-4	1	NC	1	324.36	4
85		5	max	.002	1	.006	2	0	10	3.219e-3	5	NC	1	NC	1
86			min	0	3	-.004	3	-.053	4	-4.66e-4	1	NC	1	362.685	4
87		6	max	.002	1	.005	2	0	10	3.219e-3	5	NC	1	NC	1
88			min	0	3	-.004	3	-.047	4	-4.66e-4	1	NC	1	409.901	4
89		7	max	.002	1	.005	2	0	10	3.219e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.041	4	-4.66e-4	1	NC	1	468.987	4
91		8	max	.001	1	.004	2	0	10	3.219e-3	5	NC	1	NC	1
92			min	0	3	-.003	3	-.036	4	-4.66e-4	1	NC	1	544.305	4
93		9	max	.001	1	.004	2	0	10	3.219e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.03	4	-4.66e-4	1	NC	1	642.451	4
95		10	max	.001	1	.004	2	0	10	3.219e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.025	4	-4.66e-4	1	NC	1	773.796	4
97		11	max	.001	1	.003	2	0	10	3.219e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.02	4	-4.66e-4	1	NC	1	955.419	4
99		12	max	0	1	.003	2	0	10	3.219e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.002	3	-.016	4	-4.66e-4	1	NC	1	1217.11	4
101		13	max	0	1	.002	2	0	10	3.219e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.012	4	-4.66e-4	1	NC	1	1614.751	4
103		14	max	0	1	.002	2	0	10	3.219e-3	5	NC	1	NC	1
104			min	0	3	-.001	3	-.009	4	-4.66e-4	1	NC	1	2263.603	4
105		15	max	0	1	.002	2	0	10	3.219e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.006	4	-4.66e-4	1	NC	1	3434.457	4
107		16	max	0	1	.001	2	0	10	3.219e-3	5	NC	1	NC	1
108			min	0	3	0	3	-.003	4	-4.66e-4	1	NC	1	5897.099	4
109		17	max	0	1	0	2	0	10	3.219e-3	5	NC	1	NC	1
110			min	0	3	0	3	-.002	4	-4.66e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	3.219e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-4.66e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	3.219e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-4.66e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.018	2	.003	1	1.495e-3	4	NC	3	NC	2
116			min	-.009	3	-.013	3	-.014	5	-6.478e-8	10	1642.072	2	9515.421	1
117		2	max	.008	1	.017	2	.003	1	1.517e-3	4	NC	3	NC	1
118			min	-.008	3	-.012	3	-.013	5	-6.137e-8	10	1752.238	2	NC	1
119		3	max	.007	1	.016	2	.003	1	1.538e-3	4	NC	3	NC	1
120			min	-.008	3	-.012	3	-.012	5	-5.796e-8	10	1877.877	2	NC	1
121		4	max	.007	1	.015	2	.002	1	1.56e-3	4	NC	3	NC	1
122			min	-.007	3	-.011	3	-.012	5	-5.455e-8	10	2022.078	2	NC	1
123		5	max	.006	1	.014	2	.002	1	1.582e-3	4	NC	3	NC	1
124			min	-.007	3	-.011	3	-.011	5	-8.973e-8	2	2188.824	2	NC	1
125		6	max	.006	1	.013	2	.002	1	1.604e-3	4	NC	3	NC	1
126			min	-.006	3	-.01	3	-.011	5	-1.925e-6	2	2383.324	2	NC	1
127		7	max	.006	1	.012	2	.002	1	1.625e-3	4	NC	3	NC	1
128			min	-.006	3	-.009	3	-.01	5	-3.76e-6	2	2612.53	2	NC	1
129		8	max	.005	1	.01	2	.002	1	1.647e-3	4	NC	3	NC	1
130			min	-.005	3	-.009	3	-.009	5	-6.335e-6	1	2885.925	2	NC	1
131		9	max	.005	1	.009	2	.001	1	1.669e-3	4	NC	3	NC	1
132			min	-.005	3	-.008	3	-.009	5	-1.216e-5	1	3216.792	2	NC	1
133		10	max	.004	1	.008	2	.001	1	1.69e-3	4	NC	3	NC	1
134			min	-.004	3	-.007	3	-.008	5	-1.799e-5	1	3624.317	2	NC	1
135		11	max	.004	1	.007	2	0	1	1.712e-3	4	NC	3	NC	1
136			min	-.004	3	-.006	3	-.007	5	-2.382e-5	1	4137.283	2	NC	1
137		12	max	.003	1	.006	2	0	1	1.734e-3	4	NC	3	NC	1
138			min	-.003	3	-.006	3	-.006	5	-2.964e-5	1	4800.921	2	NC	1
139		13	max	.003	1	.005	2	0	1	1.755e-3	4	NC	3	NC	1
140			min	-.003	3	-.005	3	-.005	5	-3.547e-5	1	5690.615	2	NC	1
141		14	max	.002	1	.004	2	0	1	1.777e-3	4	NC	3	NC	1
142			min	-.002	3	-.004	3	-.005	5	-4.13e-5	1	6942.053	2	NC	1
143		15	max	.002	1	.003	2	0	1	1.799e-3	4	NC	1	NC	1
144			min	-.002	3	-.003	3	-.004	5	-4.712e-5	1	8826.601	2	NC	1
145		16	max	.001	1	.003	2	0	1	1.82e-3	4	NC	1	NC	1
146			min	-.001	3	-.003	3	-.003	5	-5.295e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.842e-3	4	NC	1	NC	1
148			min	0	3	-.002	3	-.002	5	-5.877e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.864e-3	4	NC	1	NC	1
150			min	0	3	0	3	0	5	-6.46e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.885e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-7.043e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.166e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-8.582e-4	4	NC	1	NC	1
155		2	max	0	9	.001	2	.005	4	2.685e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	1	-8.449e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	9	.002	2	.009	4	2.205e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-8.315e-4	4	NC	1	NC	1
159		4	max	0	9	.003	2	.014	4	1.724e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-8.182e-4	4	NC	1	NC	1
161		5	max	0	9	.005	2	.018	4	1.244e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-8.048e-4	4	NC	1	NC	1
163		6	max	0	9	.006	2	.023	4	7.637e-6	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	-7.915e-4	4	8119.38	2	NC	1
165		7	max	0	9	.007	2	.028	4	1.808e-5	3	NC	3	NC	1
166			min	0	2	-.008	3	0	1	-7.781e-4	4	6721.369	2	NC	1
167		8	max	0	9	.008	2	.032	4	2.973e-5	3	NC	3	NC	1
168			min	0	2	-.009	3	-.001	1	-7.648e-4	4	5693.969	2	NC	1
169		9	max	0	9	.009	2	.037	4	4.139e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	-.001	1	-7.514e-4	4	4903.087	2	NC	1
171		10	max	0	9	.011	2	.041	4	5.304e-5	3	NC	3	NC	1
172			min	-.001	2	-.012	3	-.001	1	-7.381e-4	4	4274.315	2	NC	1
173		11	max	0	9	.012	2	.045	4	6.47e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	-.001	1	-7.247e-4	4	3762.836	2	NC	1
175		12	max	0	9	.014	2	.05	4	7.635e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-7.114e-4	4	3339.82	2	NC	1
177		13	max	.001	9	.015	2	.054	4	8.8e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.002	1	-6.98e-4	4	2985.685	2	NC	1
179		14	max	.001	9	.017	2	.058	4	9.966e-5	3	NC	3	NC	1
180			min	-.001	2	-.016	3	-.002	1	-6.847e-4	4	2686.515	2	NC	1
181		15	max	.001	9	.019	2	.062	4	1.113e-4	3	NC	3	NC	1
182			min	-.002	2	-.017	3	-.002	1	-6.713e-4	4	2432.052	2	NC	1
183		16	max	.001	9	.021	2	.065	4	1.23e-4	3	NC	3	NC	1
184			min	-.002	2	-.017	3	-.002	1	-6.58e-4	4	2214.499	2	NC	1
185		17	max	.001	9	.023	2	.069	4	1.346e-4	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-6.446e-4	4	2027.8	2	NC	1
187		18	max	.001	9	.025	2	.073	4	1.463e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	-.002	1	-6.313e-4	4	1867.16	2	NC	1
189		19	max	.002	9	.027	2	.076	4	1.579e-4	3	NC	3	NC	1
190			min	-.002	2	-.019	3	-.002	1	-6.179e-4	4	1728.74	2	NC	1
191	M8	1	max	.007	1	.021	2	.002	1	2.98e-3	4	NC	1	NC	1
192			min	-.002	3	-.015	3	-.08	4	-1.257e-4	3	NC	1	240.608	4
193		2	max	.006	1	.02	2	.002	1	2.98e-3	4	NC	1	NC	1
194			min	-.002	3	-.014	3	-.074	4	-1.257e-4	3	NC	1	262.289	4
195		3	max	.006	1	.019	2	.001	1	2.98e-3	4	NC	1	NC	1
196			min	-.002	3	-.013	3	-.067	4	-1.257e-4	3	NC	1	288.095	4
197		4	max	.006	1	.018	2	.001	1	2.98e-3	4	NC	1	NC	1
198			min	-.002	3	-.012	3	-.061	4	-1.257e-4	3	NC	1	319.11	4
199		5	max	.005	1	.017	2	.001	1	2.98e-3	4	NC	1	NC	1
200			min	-.001	3	-.011	3	-.054	4	-1.257e-4	3	NC	1	356.816	4
201		6	max	.005	1	.015	2	.001	1	2.98e-3	4	NC	1	NC	1
202			min	-.001	3	-.01	3	-.048	4	-1.257e-4	3	NC	1	403.269	4
203		7	max	.005	1	.014	2	0	1	2.98e-3	4	NC	1	NC	1
204			min	-.001	3	-.01	3	-.042	4	-1.257e-4	3	NC	1	461.4	4
205		8	max	.004	1	.013	2	0	1	2.98e-3	4	NC	1	NC	1
206			min	-.001	3	-.009	3	-.036	4	-1.257e-4	3	NC	1	535.5	4
207		9	max	.004	1	.012	2	0	1	2.98e-3	4	NC	1	NC	1
208			min	-.001	3	-.008	3	-.031	4	-1.257e-4	3	NC	1	632.061	4
209		10	max	.003	1	.011	2	0	1	2.98e-3	4	NC	1	NC	1
210			min	0	3	-.007	3	-.025	4	-1.257e-4	3	NC	1	761.284	4
211		11	max	.003	1	.009	2	0	1	2.98e-3	4	NC	1	NC	1
212			min	0	3	-.006	3	-.021	4	-1.257e-4	3	NC	1	939.973	4
213		12	max	.003	1	.008	2	0	1	2.98e-3	4	NC	1	NC	1





Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	1	0	2	.007	4	2.985e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-7.59e-4	4	NC	1	6878.66	4
273		4	max	0	1	0	2	.01	4	1.767e-5	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-8.309e-4	4	NC	1	4532.724	4
275		5	max	0	1	0	2	.014	4	5.486e-6	3	NC	1	NC	1
276			min	0	10	-.003	3	0	3	-9.027e-4	4	NC	1	3366.374	4
277		6	max	0	1	0	2	.017	4	-4.589e-6	12	NC	1	NC	1
278			min	0	10	-.004	3	-.001	3	-9.746e-4	4	NC	1	2671.012	4
279		7	max	0	1	0	2	.021	5	-1.199e-5	12	NC	1	NC	1
280			min	0	10	-.004	3	-.001	3	-1.046e-3	4	NC	1	2209.851	5
281		8	max	0	1	.001	2	.024	5	-1.365e-5	10	NC	1	NC	1
282			min	0	10	-.005	3	-.001	1	-1.118e-3	4	NC	1	1879.565	5
283		9	max	0	1	.002	2	.028	5	-1.515e-5	10	NC	1	NC	1
284			min	0	10	-.005	3	-.002	1	-1.19e-3	4	NC	1	1633.692	5
285		10	max	0	1	.002	2	.032	5	-1.665e-5	10	NC	1	NC	1
286			min	0	10	-.006	3	-.002	1	-1.262e-3	4	NC	1	1443.74	5
287		11	max	0	1	.002	2	.036	5	-1.815e-5	10	NC	1	NC	1
288			min	0	10	-.006	3	-.003	1	-1.334e-3	4	NC	1	1292.632	5
289		12	max	0	1	.003	2	.039	5	-1.965e-5	10	NC	1	NC	1
290			min	0	10	-.007	3	-.004	1	-1.406e-3	4	NC	1	1169.516	5
291		13	max	0	1	.004	2	.043	5	-2.115e-5	10	NC	1	NC	1
292			min	0	10	-.007	3	-.004	1	-1.478e-3	4	NC	1	1067.166	5
293		14	max	0	1	.005	2	.047	5	-2.264e-5	10	NC	1	NC	2
294			min	0	10	-.007	3	-.005	1	-1.549e-3	4	NC	1	980.589	5
295		15	max	0	1	.005	2	.051	5	-2.414e-5	10	NC	3	NC	2
296			min	0	10	-.007	3	-.005	1	-1.621e-3	4	8529.698	2	906.225	5
297		16	max	0	1	.006	2	.055	5	-2.564e-5	10	NC	3	NC	2
298			min	0	10	-.007	3	-.006	1	-1.693e-3	4	7280.819	2	841.469	5
299		17	max	0	1	.007	2	.059	5	-2.714e-5	10	NC	3	NC	2
300			min	0	10	-.007	3	-.006	1	-1.765e-3	4	6302.26	2	784.374	5
301		18	max	0	1	.008	2	.063	5	-2.864e-5	10	NC	3	NC	2
302			min	0	10	-.007	3	-.007	1	-1.837e-3	4	5528.726	2	733.46	5
303		19	max	0	1	.009	2	.067	5	-3.014e-5	10	NC	3	NC	2
304			min	0	10	-.007	3	-.007	1	-1.909e-3	4	4913.022	2	687.584	5
305	M12	1	max	.002	1	.007	2	.006	1	4.079e-3	4	NC	1	NC	2
306			min	0	3	-.005	3	-.074	5	2.921e-5	10	NC	1	262.745	5
307		2	max	.002	1	.007	2	.005	1	4.079e-3	4	NC	1	NC	2
308			min	0	3	-.005	3	-.067	5	2.921e-5	10	NC	1	286.416	5
309		3	max	.002	1	.006	2	.005	1	4.079e-3	4	NC	1	NC	2
310			min	0	3	-.005	3	-.061	5	2.921e-5	10	NC	1	314.589	5
311		4	max	.002	1	.006	2	.005	1	4.079e-3	4	NC	1	NC	2
312			min	0	3	-.005	3	-.055	5	2.921e-5	10	NC	1	348.449	5
313		5	max	.002	1	.006	2	.004	1	4.079e-3	4	NC	1	NC	2
314			min	0	3	-.004	3	-.05	5	2.921e-5	10	NC	1	389.613	5
315		6	max	.002	1	.005	2	.004	1	4.079e-3	4	NC	1	NC	2
316			min	0	3	-.004	3	-.044	5	2.921e-5	10	NC	1	440.325	5
317		7	max	.002	1	.005	2	.003	1	4.079e-3	4	NC	1	NC	2
318			min	0	3	-.004	3	-.038	5	2.921e-5	10	NC	1	503.785	5
319		8	max	.001	1	.004	2	.003	1	4.079e-3	4	NC	1	NC	2
320			min	0	3	-.003	3	-.033	5	2.921e-5	10	NC	1	584.677	5
321		9	max	.001	1	.004	2	.002	1	4.079e-3	4	NC	1	NC	2
322			min	0	3	-.003	3	-.028	5	2.921e-5	10	NC	1	690.086	5
323		10	max	.001	1	.004	2	.002	1	4.079e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.023	5	2.921e-5	10	NC	1	831.149	5
325		11	max	.001	1	.003	2	.002	1	4.079e-3	4	NC	1	NC	1
326			min	0	3	-.002	3	-.019	5	2.921e-5	10	NC	1	1026.206	5
327		12	max	0	1	.003	2	.001	1	4.079e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.015	5	2.921e-5	10	NC	1	1307.252	5
329		13	max	0	1	.002	2	0	1	4.079e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.011	5	2.921e-5	10	NC	1	1734.295	5
331		14	max	0	1	.002	2	0	1	4.079e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.008	5	2.921e-5	10	NC	1	2431.115	5
333		15	max	0	1	.002	2	0	1	4.079e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.005	5	2.921e-5	10	NC	1	3688.508	5
335		16	max	0	1	.001	2	0	1	4.079e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.003	5	2.921e-5	10	NC	1	6333.128	5
337		17	max	0	1	0	2	0	1	4.079e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.001	5	2.921e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.079e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	2.921e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.079e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	2.921e-5	10	NC	1	NC	1
343	M1	1	max	.005	3	.023	3	.007	5	9.831e-3	1	NC	1	NC	1
344			min	-.006	2	-.026	1	-.003	1	-9.72e-3	3	NC	1	NC	1
345		2	max	.005	3	.012	3	.01	5	4.704e-3	1	NC	4	NC	1
346			min	-.006	2	-.014	1	-.006	1	-4.794e-3	3	3740.392	1	NC	1
347		3	max	.005	3	.002	3	.014	5	3.189e-4	5	NC	4	NC	1
348			min	-.006	2	-.002	1	-.007	1	-3.276e-4	1	1934.801	1	7222.929	5
349		4	max	.005	3	.008	1	.018	5	3.141e-4	5	NC	5	NC	2
350			min	-.006	2	-.006	3	-.008	1	-2.729e-4	1	1368.019	1	4554.989	5
351		5	max	.005	3	.016	1	.022	5	3.092e-4	5	NC	5	NC	2
352			min	-.006	2	-.012	3	-.009	1	-2.182e-4	1	1095.479	1	3257.005	5
353		6	max	.005	3	.023	1	.026	5	3.044e-4	5	NC	5	NC	2
354			min	-.006	2	-.018	3	-.008	1	-1.636e-4	1	941.359	1	2500.574	5
355		7	max	.005	3	.028	1	.031	5	2.995e-4	5	NC	5	NC	1
356			min	-.006	2	-.022	3	-.007	1	-1.089e-4	1	847.978	1	2011.412	5
357		8	max	.005	3	.032	1	.035	5	2.947e-4	5	NC	5	NC	1
358			min	-.006	2	-.024	3	-.006	1	-5.427e-5	1	791.341	1	1672.766	5
359		9	max	.005	3	.034	1	.04	5	2.906e-4	4	NC	5	NC	1
360			min	-.006	2	-.026	3	-.004	1	3.927e-7	1	760.242	1	1420.252	4
361		10	max	.005	3	.035	1	.045	5	2.971e-4	4	NC	5	NC	1
362			min	-.006	2	-.026	3	-.003	1	1.031e-5	10	749.502	1	1224.006	4
363		11	max	.005	3	.034	1	.05	4	3.036e-4	4	NC	5	NC	1
364			min	-.006	2	-.025	3	0	1	1.336e-5	10	757.533	1	1074.764	4
365		12	max	.005	3	.032	1	.056	4	3.101e-4	4	NC	5	NC	1
366			min	-.006	2	-.023	3	0	10	1.641e-5	10	785.683	1	958.768	4
367		13	max	.005	3	.028	1	.061	4	3.166e-4	4	NC	5	NC	1
368			min	-.006	2	-.02	3	0	10	1.947e-5	10	838.816	1	867.097	4
369		14	max	.005	3	.022	1	.066	4	3.231e-4	4	NC	5	NC	2
370			min	-.006	2	-.016	3	0	10	2.252e-5	10	927.62	1	793.755	4
371		15	max	.005	3	.015	1	.07	4	3.296e-4	4	NC	5	NC	2
372			min	-.006	2	-.011	3	0	10	2.514e-5	12	1075.04	1	734.59	4
373		16	max	.005	3	.007	1	.075	4	5.772e-4	4	NC	5	NC	2
374			min	-.006	2	-.005	3	0	10	2.496e-5	12	1335.881	1	686.66	4
375		17	max	.005	3	.002	3	.079	4	6.564e-3	4	NC	4	NC	1
376			min	-.006	2	-.003	1	0	10	1.33e-5	10	1875.52	1	647.881	4
377		18	max	.005	3	.009	3	.083	4	5.625e-3	1	NC	4	NC	1
378			min	-.006	2	-.016	1	0	10	-2.361e-3	3	3613.522	1	616.636	4
379		19	max	.005	3	.017	3	.086	4	1.129e-2	1	NC	1	NC	1
380			min	-.006	2	-.029	1	-.002	1	-4.798e-3	3	NC	1	592.45	4
381	M5	1	max	.013	3	.065	3	.007	5	5.542e-6	4	NC	1	NC	1
382			min	-.018	2	-.075	1	-.003	1	3.995e-8	2	NC	1	NC	1
383		2	max	.013	3	.035	3	.01	5	1.493e-4	5	NC	5	NC	1
384			min	-.019	2	-.04	1	-.003	1	-7.4e-5	1	1325.898	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.013	3	.008	3	.014	5	2.908e-4	5	NC	5	NC	1
386			min	-.019	2	-.007	1	-.003	1	-1.466e-4	1	682.256	1	NC	1
387		4	max	.013	3	.021	1	.018	5	3.042e-4	5	NC	5	NC	1
388			min	-.019	2	-.015	3	-.003	1	-1.389e-4	1	481.383	1	NC	1
389		5	max	.013	3	.045	1	.022	5	3.177e-4	5	NC	5	NC	1
390			min	-.019	2	-.034	3	-.003	1	-1.312e-4	1	384.812	1	NC	1
391		6	max	.013	3	.065	1	.027	5	3.311e-4	5	NC	5	NC	1
392			min	-.019	2	-.048	3	-.003	1	-1.235e-4	1	330.161	1	NC	1
393		7	max	.013	3	.08	1	.032	5	3.445e-4	5	NC	15	NC	1
394			min	-.019	2	-.059	3	-.003	1	-1.158e-4	1	296.983	1	NC	1
395		8	max	.013	3	.091	1	.037	5	3.58e-4	5	NC	15	NC	1
396			min	-.019	2	-.067	3	-.003	1	-1.081e-4	1	276.779	1	NC	1
397		9	max	.013	3	.098	1	.042	5	3.714e-4	5	NC	15	NC	1
398			min	-.019	2	-.071	3	-.003	1	-1.004e-4	1	265.573	1	NC	1
399		10	max	.013	3	.1	1	.048	5	3.849e-4	5	NC	15	NC	1
400			min	-.019	2	-.071	3	-.002	1	-9.275e-5	1	261.523	1	NC	1
401		11	max	.013	3	.098	1	.053	4	3.983e-4	5	NC	15	NC	1
402			min	-.019	2	-.068	3	-.002	1	-8.506e-5	1	264.052	1	NC	1
403		12	max	.013	3	.091	1	.058	4	4.117e-4	5	NC	15	NC	1
404			min	-.019	2	-.063	3	-.002	1	-7.738e-5	1	273.616	1	NC	1
405		13	max	.013	3	.08	1	.063	4	4.252e-4	5	NC	15	NC	1
406			min	-.019	2	-.054	3	-.002	1	-6.969e-5	1	291.902	1	NC	1
407		14	max	.013	3	.064	1	.068	4	4.386e-4	5	NC	5	NC	1
408			min	-.019	2	-.043	3	-.002	1	-6.2e-5	1	322.639	1	NC	1
409		15	max	.013	3	.044	1	.073	4	4.52e-4	5	NC	5	NC	1
410			min	-.019	2	-.029	3	-.002	1	-5.432e-5	1	373.859	1	NC	1
411		16	max	.014	3	.019	1	.077	4	7.018e-4	5	NC	5	NC	1
412			min	-.019	2	-.013	3	-.002	1	-5.016e-5	1	464.836	1	NC	1
413		17	max	.014	3	.005	3	.08	4	6.615e-3	4	NC	5	NC	1
414			min	-.019	2	-.011	1	-.002	1	-1.3e-4	1	654.46	1	NC	1
415		18	max	.014	3	.026	3	.083	4	3.395e-3	4	NC	5	NC	1
416			min	-.019	2	-.046	1	-.002	1	-6.635e-5	1	1268.013	1	NC	1
417		19	max	.014	3	.047	3	.086	4	2.244e-6	5	NC	1	NC	1
418			min	-.019	2	-.083	1	-.002	1	-1.437e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.023	3	.006	5	9.722e-3	3	NC	1	NC	1
420			min	-.006	2	-.026	1	-.004	1	-9.831e-3	1	NC	1	NC	1
421		2	max	.005	3	.012	3	.005	5	4.827e-3	3	NC	4	NC	1
422			min	-.006	2	-.014	1	0	1	-4.857e-3	1	3741.736	1	NC	1
423		3	max	.005	3	.002	3	.005	4	2.532e-5	2	NC	4	NC	1
424			min	-.006	2	-.002	1	0	3	-5.384e-6	5	1935.521	1	NC	1
425		4	max	.005	3	.008	1	.007	4	1.337e-5	3	NC	5	NC	1
426			min	-.006	2	-.006	3	0	3	-3.154e-5	4	1368.536	1	NC	1
427		5	max	.005	3	.016	1	.009	4	5.088e-6	10	NC	5	NC	1
428			min	-.006	2	-.012	3	-.001	3	-5.943e-5	4	1095.884	1	NC	1
429		6	max	.005	3	.023	1	.012	4	2.029e-6	10	NC	5	NC	1
430			min	-.006	2	-.018	3	-.002	3	-9.856e-5	1	941.695	1	7109.072	4
431		7	max	.005	3	.028	1	.015	4	-1.029e-6	10	NC	5	NC	1
432			min	-.006	2	-.022	3	-.002	3	-1.395e-4	1	848.267	1	4604.127	4
433		8	max	.005	3	.032	1	.02	4	-4.088e-6	10	NC	5	NC	1
434			min	-.006	2	-.024	3	-.002	3	-1.804e-4	1	791.598	1	3259.162	4
435		9	max	.005	3	.034	1	.024	4	-7.147e-6	10	NC	5	NC	1
436			min	-.006	2	-.026	3	-.002	3	-2.214e-4	1	760.475	1	2451.097	4
437		10	max	.005	3	.035	1	.03	5	-1.021e-5	10	NC	5	NC	1
438			min	-.006	2	-.026	3	-.002	3	-2.623e-4	1	749.718	1	1926.427	4
439		11	max	.005	3	.034	1	.035	5	-1.326e-5	10	NC	5	NC	1
440			min	-.006	2	-.025	3	-.004	1	-3.033e-4	1	757.737	1	1565.853	4
441		12	max	.005	3	.032	1	.042	5	-1.632e-5	10	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.006	2	-.023	3	-.005	1	-3.442e-4	1	785.88	1	1307.093	4
443		13	max	.005	3	.028	1	.048	5	-1.938e-5	10	NC	5	NC	2
444			min	-.006	2	-.02	3	-.006	1	-3.851e-4	1	839.011	1	1114.992	4
445		14	max	.005	3	.022	1	.054	5	-2.244e-5	10	NC	5	NC	2
446			min	-.006	2	-.016	3	-.007	1	-4.261e-4	1	927.818	1	967.578	5
447		15	max	.005	3	.015	1	.061	5	-2.55e-5	10	NC	5	NC	2
448			min	-.006	2	-.011	3	-.007	1	-4.67e-4	1	1075.25	1	853.575	5
449		16	max	.005	3	.007	1	.067	5	-6.416e-6	15	NC	5	NC	2
450			min	-.006	2	-.005	3	-.007	1	-4.995e-4	1	1336.121	1	763.487	4
451		17	max	.005	3	.002	3	.073	5	6.356e-3	5	NC	4	NC	1
452			min	-.006	2	-.003	1	-.006	1	-3.308e-4	1	1875.834	1	690.271	4
453		18	max	.005	3	.009	3	.079	5	3.095e-3	5	NC	4	NC	1
454			min	-.006	2	-.016	1	-.004	1	-5.76e-3	1	3614.101	1	630.29	4
455		19	max	.005	3	.017	3	.086	4	4.797e-3	3	NC	1	NC	1
456			min	-.006	2	-.029	1	-.001	1	-1.129e-2	1	NC	1	581.261	4
457	M13	1	max	.004	1	.023	3	.005	3	4.023e-3	3	NC	1	NC	1
458			min	-.006	5	-.026	1	-.006	2	-4.724e-3	1	NC	1	NC	1
459		2	max	.004	1	.101	3	.012	1	4.813e-3	3	NC	4	NC	2
460			min	-.006	5	-.105	1	-.002	10	-5.661e-3	1	1895.879	1	8914.348	1
461		3	max	.004	1	.165	3	.034	1	5.603e-3	3	NC	5	NC	2
462			min	-.006	5	-.171	1	-.004	5	-6.597e-3	1	1037.817	1	3924.323	1
463		4	max	.003	1	.207	3	.051	1	6.393e-3	3	NC	5	NC	3
464			min	-.006	5	-.213	1	-.007	5	-7.534e-3	1	802.26	1	2706.176	1
465		5	max	.003	1	.221	3	.058	1	7.183e-3	3	NC	5	NC	3
466			min	-.006	5	-.228	1	-.009	5	-8.471e-3	1	742.605	1	2383.521	1
467		6	max	.003	1	.208	3	.054	1	7.974e-3	3	NC	5	NC	3
468			min	-.006	5	-.216	1	-.011	5	-9.407e-3	1	789.522	1	2565.398	1
469		7	max	.003	1	.174	3	.038	1	8.764e-3	3	NC	5	NC	2
470			min	-.007	5	-.182	1	-.011	5	-1.034e-2	1	960.375	1	3485.955	1
471		8	max	.003	1	.127	3	.016	1	9.554e-3	3	NC	5	NC	2
472			min	-.007	5	-.137	1	-.011	5	-1.128e-2	1	1355.556	1	7195.988	1
473		9	max	.003	1	.085	3	.012	3	1.034e-2	3	NC	4	NC	1
474			min	-.007	5	-.094	1	-.014	2	-1.222e-2	1	2196.713	1	NC	1
475		10	max	.003	1	.065	3	.013	3	1.113e-2	3	NC	4	NC	1
476			min	-.007	5	-.075	1	-.018	2	-1.315e-2	1	3073.594	1	NC	1
477		11	max	.003	1	.085	3	.015	3	1.034e-2	3	NC	4	NC	1
478			min	-.007	5	-.094	1	-.014	2	-1.222e-2	1	2196.714	1	NC	1
479		12	max	.003	1	.128	3	.017	1	9.555e-3	3	NC	5	NC	2
480			min	-.007	5	-.137	1	-.007	10	-1.128e-2	1	1355.556	1	7098.359	1
481		13	max	.003	1	.174	3	.039	1	8.765e-3	3	NC	5	NC	2
482			min	-.007	5	-.182	1	-.004	10	-1.034e-2	1	960.375	1	3468.035	1
483		14	max	.003	1	.208	3	.054	1	7.975e-3	3	NC	5	NC	3
484			min	-.007	5	-.216	1	-.002	10	-9.408e-3	1	789.523	1	2561.694	1
485		15	max	.003	1	.221	3	.058	1	7.186e-3	3	NC	5	NC	3
486			min	-.007	5	-.228	1	0	10	-8.471e-3	1	742.605	1	2386.478	1
487		16	max	.003	1	.207	3	.051	1	6.396e-3	3	NC	5	NC	3
488			min	-.007	5	-.213	1	-.004	5	-7.535e-3	1	802.261	1	2716.785	1
489		17	max	.003	1	.165	3	.033	1	5.606e-3	3	NC	5	NC	2
490			min	-.007	5	-.171	1	-.006	5	-6.598e-3	1	1037.818	1	3953.501	1
491		18	max	.003	1	.101	3	.012	1	4.817e-3	3	NC	4	NC	2
492			min	-.007	5	-.105	1	-.005	5	-5.662e-3	1	1895.88	1	9035.819	1
493		19	max	.003	1	.023	3	.005	3	4.027e-3	3	NC	1	NC	1
494			min	-.007	5	-.026	1	-.006	2	-4.725e-3	1	NC	1	NC	1
495	M16	1	max	.001	1	.017	3	.005	3	4.998e-3	1	NC	1	NC	1
496			min	-.086	4	-.029	1	-.006	2	-3.044e-3	3	NC	1	NC	1
497		2	max	.001	1	.056	3	.016	4	6.009e-3	1	NC	4	NC	2
498			min	-.086	4	-.12	1	-.002	10	-3.608e-3	3	1650.421	1	8933.309	4



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Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.001	1	.088	3	.033	1	7.02e-3	1	NC	5	NC	2
500		min	-.086	4	-.195	1	-.001	10	-4.172e-3	3	903.679	1	3935.255	1
501	4	max	.001	1	.109	3	.05	1	8.031e-3	1	NC	5	NC	3
502		min	-.086	4	-.243	1	0	10	-4.736e-3	3	698.885	1	2714.569	1
503	5	max	.001	1	.117	3	.058	1	9.042e-3	1	NC	5	NC	3
504		min	-.086	4	-.26	1	0	10	-5.3e-3	3	647.402	1	2392.676	1
505	6	max	.002	1	.112	3	.053	1	1.005e-2	1	NC	5	NC	3
506		min	-.086	4	-.246	1	-.002	10	-5.864e-3	3	689.173	1	2579.141	1
507	7	max	.002	1	.097	3	.038	1	1.106e-2	1	NC	5	NC	2
508		min	-.086	4	-.207	1	-.004	10	-6.428e-3	3	840.198	1	3517.079	1
509	8	max	.002	1	.076	3	.016	3	1.207e-2	1	NC	5	NC	2
510		min	-.086	4	-.155	1	-.007	10	-6.992e-3	3	1191.165	1	7353.176	1
511	9	max	.002	1	.056	3	.015	3	1.309e-2	1	NC	4	NC	1
512		min	-.086	4	-.106	1	-.015	2	-7.556e-3	3	1947.631	1	NC	1
513	10	max	.002	1	.047	3	.014	3	1.41e-2	1	NC	4	NC	1
514		min	-.086	4	-.083	1	-.019	2	-8.12e-3	3	2750.349	1	NC	1
515	11	max	.002	1	.056	3	.013	3	1.309e-2	1	NC	4	NC	1
516		min	-.086	4	-.106	1	-.015	2	-7.555e-3	3	1947.631	1	NC	1
517	12	max	.002	1	.076	3	.016	1	1.208e-2	1	NC	5	NC	2
518		min	-.086	4	-.155	1	-.007	10	-6.991e-3	3	1191.165	1	7296.515	1
519	13	max	.002	1	.097	3	.038	1	1.106e-2	1	NC	5	NC	2
520		min	-.086	4	-.207	1	-.004	10	-6.427e-3	3	840.198	1	3512.349	1
521	14	max	.002	1	.112	3	.053	1	1.005e-2	1	NC	5	NC	3
522		min	-.086	4	-.246	1	-.002	10	-5.862e-3	3	689.173	1	2583.524	1
523	15	max	.002	1	.117	3	.058	1	9.042e-3	1	NC	5	NC	3
524		min	-.086	4	-.26	1	-.004	5	-5.298e-3	3	647.402	1	2402.684	1
525	16	max	.002	1	.109	3	.05	1	8.032e-3	1	NC	5	NC	3
526		min	-.086	4	-.243	1	-.008	5	-4.734e-3	3	698.885	1	2733.429	1
527	17	max	.002	1	.088	3	.033	1	7.021e-3	1	NC	5	NC	2
528		min	-.086	4	-.195	1	-.009	5	-4.169e-3	3	903.68	1	3978.074	1
529	18	max	.002	1	.056	3	.012	1	6.01e-3	1	NC	4	NC	2
530		min	-.086	4	-.12	1	-.007	5	-3.605e-3	3	1650.422	1	9102.346	1
531	19	max	.002	1	.017	3	.005	3	4.999e-3	1	NC	1	NC	1
532		min	-.086	4	-.029	1	-.006	2	-3.041e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	2.998e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.403e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.007	4	7.746e-4	3	NC	1	NC	1
536		min	0	5	-.007	1	0	3	-6.496e-4	1	NC	1	NC	1
537	3	max	0	3	0	15	.014	4	1.249e-3	3	NC	5	NC	1
538		min	-.001	5	-.015	1	-.003	3	-1.249e-3	1	5661.417	2	5720.136	4
539	4	max	0	3	-.001	15	.021	4	1.724e-3	3	NC	5	NC	9
540		min	-.002	5	-.021	1	-.006	3	-1.848e-3	1	3884.063	2	3775.317	4
541	5	max	0	3	-.001	15	.028	4	2.199e-3	3	NC	5	NC	9
542		min	-.003	5	-.027	1	-.009	3	-2.447e-3	1	3030.774	2	2885.54	4
543	6	max	0	3	-.002	15	.033	4	2.674e-3	3	NC	5	9234.679	10
544		min	-.004	5	-.033	1	-.014	3	-3.047e-3	1	2550.716	2	2415.138	4
545	7	max	0	3	-.002	15	.037	4	3.149e-3	3	NC	5	7243.185	10
546		min	-.004	5	-.037	1	-.018	3	-3.646e-3	1	2262.025	2	2159.699	4
547	8	max	0	3	-.002	15	.039	4	3.623e-3	3	NC	5	5987.387	10
548		min	-.005	5	-.04	1	-.022	3	-4.245e-3	1	2088.767	2	2038.434	4
549	9	max	0	3	-.002	15	.04	4	4.098e-3	3	NC	5	5163.983	10
550		min	-.006	5	-.042	1	-.026	3	-4.844e-3	1	1995.509	2	2019.227	4
551	10	max	0	3	-.002	15	.038	4	4.573e-3	3	NC	5	4620.108	10
552		min	-.006	5	-.043	1	-.029	3	-5.444e-3	1	1966.007	2	1917.321	1
553	11	max	0	3	-.002	15	.036	1	5.048e-3	3	NC	5	4275.386	10
554		min	-.007	5	-.043	1	-.031	3	-6.043e-3	1	1995.509	2	1767.631	1
555	12	max	0	3	-.001	15	.038	1	5.523e-3	3	NC	5	5028.736	15





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Software
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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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