



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

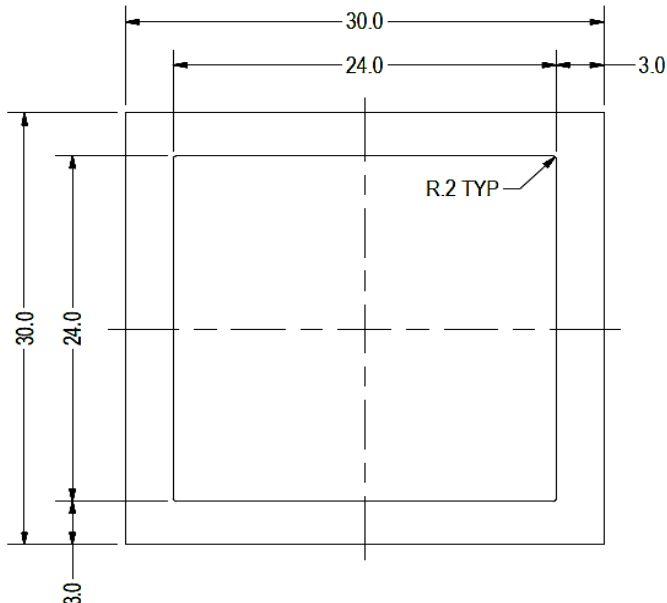
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

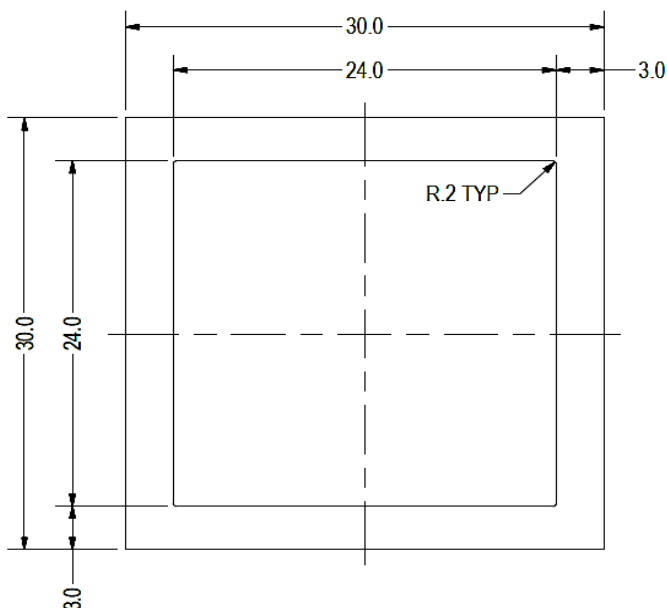
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.055 k-ft
P_n =	0.254 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	15%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.789 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	21%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

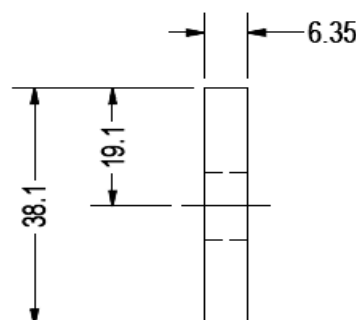
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.902 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<u>20%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.007 k-ft
P_n =	0.258 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>18%</u>



A cross brace kit is required every 11 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

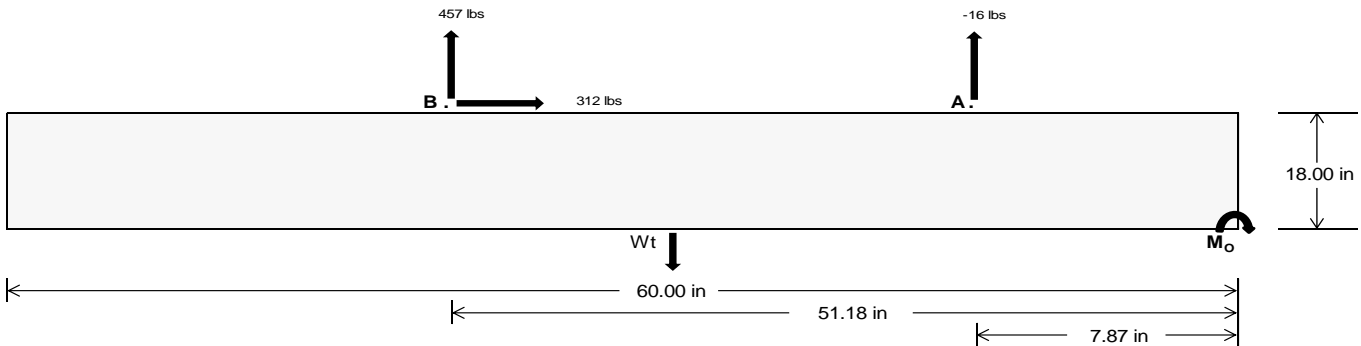
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>22.18</u>	<u>1903.94</u>	k
Compressive Load =	<u>1364.88</u>	<u>1415.96</u>	k
Lateral Load =	<u>44.80</u>	<u>1298.80</u>	k
Moment (Weak Axis) =	<u>0.07</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 28874.0$ in-lbs
Resisting Force Required = 962.47 lbs
S.F. = 1.67
Weight Required = 1604.11 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 312.03 lbs
Friction = 0.4
Weight Required = 780.08 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 312.03 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	548 lbs	548 lbs	548 lbs	548 lbs	381 lbs	381 lbs	381 lbs	381 lbs	642 lbs	642 lbs	642 lbs	642 lbs	32 lbs	32 lbs	32 lbs	32 lbs
F_B	372 lbs	372 lbs	372 lbs	372 lbs	607 lbs	607 lbs	607 lbs	607 lbs	696 lbs	696 lbs	696 lbs	696 lbs	-914 lbs	-914 lbs	-914 lbs	-914 lbs
F_V	74 lbs	74 lbs	74 lbs	74 lbs	574 lbs	574 lbs	574 lbs	574 lbs	479 lbs	479 lbs	479 lbs	479 lbs	-624 lbs	-624 lbs	-624 lbs	-624 lbs
P_{total}	2914 lbs	3005 lbs	3096 lbs	3186 lbs	2982 lbs	3072 lbs	3163 lbs	3253 lbs	3332 lbs	3423 lbs	3513 lbs	3604 lbs	315 lbs	369 lbs	424 lbs	478 lbs
M	466 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	491 lbs-ft	491 lbs-ft	491 lbs-ft	491 lbs-ft	674 lbs-ft	674 lbs-ft	674 lbs-ft	674 lbs-ft	736 lbs-ft	736 lbs-ft	736 lbs-ft	736 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.16 ft	0.16 ft	0.16 ft	0.15 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.34 ft	2.00 ft	1.74 ft	1.54 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	257.0 psf	255.3 psf	253.7 psf	252.2 psf	261.0 psf	259.1 psf	257.4 psf	255.8 psf	275.3 psf	272.8 psf	270.5 psf	268.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	378.9 psf	371.9 psf	365.4 psf	359.5 psf	389.5 psf	382.0 psf	375.2 psf	368.9 psf	451.7 psf	441.5 psf	432.2 psf	423.6 psf	713.8 psf	254.3 psf	185.5 psf	159.5 psf

Maximum Bearing Pressure = 714 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

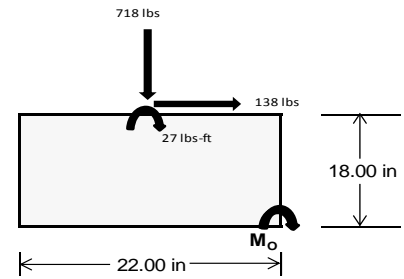
Overturning Check

$M_o = 424.4 \text{ ft-lbs}$
 Resisting Force Required = 462.93 lbs
 S.F. = 1.67
 Weight Required = 771.55 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	154 lbs	162 lbs	92 lbs	327 lbs	718 lbs	278 lbs	90 lbs	-1 lbs	30 lbs
F_v	23 lbs	182 lbs	25 lbs	15 lbs	138 lbs	19 lbs	24 lbs	182 lbs	24 lbs
P_{total}	2623 lbs	2630 lbs	2560 lbs	2677 lbs	3068 lbs	2628 lbs	811 lbs	721 lbs	752 lbs
M	67 lbs-ft	311 lbs-ft	74 lbs-ft	43 lbs-ft	234 lbs-ft	59 lbs-ft	69 lbs-ft	310 lbs-ft	73 lbs-ft
e	0.03 ft	0.12 ft	0.03 ft	0.02 ft	0.08 ft	0.02 ft	0.09 ft	0.43 ft	0.10 ft
$L/6$	0.31 ft	1.60 ft	1.78 ft	1.80 ft	1.68 ft	1.79 ft	1.66 ft	0.97 ft	1.64 ft
f_{min}	262.1 sqft	176.0 sqft	252.9 sqft	276.6 sqft	251.1 sqft	265.7 sqft	63.8 sqft	-32.1 sqft	56.2 sqft
f_{max}	310.1 psf	397.8 psf	305.6 psf	307.4 psf	418.3 psf	307.7 psf	113.2 psf	189.3 psf	108.0 psf



Maximum Bearing Pressure = 418 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.403 k
Allowable Uplift =	1.214 k
Utilization =	<u>33%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.107 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.050 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Diagonal Strut

Maximum Axial Load =	0.789 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.197 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.258 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.117 in
	<u>0.117 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.255$$

$$242.167$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.255$$

$$251.476$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.2$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.3 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.203 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &22.2752 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.453 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

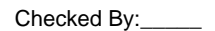
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 35° 85mph 30psf 7.75ft 7-05.rdb Page 20



RISA-3D Version 13.0.0 \.....\PVMMini 60 Cell 1V 35° 85mph 30psf 7.75ft 7-05.rdb Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	277.607	1	-.031	15	.571	1	0	12	.002	4	0	15
30			min	-365.479	3	-.129	4	-.369	5	-.001	1	0	3	0	6
31		16	max	277.742	1	-.044	15	.571	1	0	12	.002	4	0	15
32			min	-365.378	3	-.187	4	-.492	5	-.001	1	0	3	0	6
33		17	max	277.876	1	-.058	15	.571	1	0	12	.001	4	0	15
34			min	-365.277	3	-.244	4	-.615	5	-.001	1	0	3	0	6
35		18	max	278.011	1	-.071	15	.571	1	0	12	.001	1	0	15
36			min	-365.176	3	-.302	4	-.738	5	-.001	1	0	3	0	6
37		19	max	278.146	1	-.085	15	.571	1	0	12	.002	1	0	15
38			min	-365.075	3	-.359	4	-.862	5	-.001	1	0	3	0	6
39	M3	1	max	196.944	2	1.733	6	-.041	12	0	5	.002	1	0	6
40			min	-218.474	3	.407	15	-1.44	4	0	1	0	12	0	15
41		2	max	196.874	2	1.557	6	-.041	12	0	5	.002	1	0	2
42			min	-218.526	3	.366	15	-1.307	4	0	1	0	12	0	3
43		3	max	196.804	2	1.381	6	-.041	12	0	5	.002	1	0	2
44			min	-218.579	3	.324	15	-1.173	4	0	1	0	15	0	3
45		4	max	196.734	2	1.204	6	-.041	12	0	5	.002	1	0	15
46			min	-218.631	3	.283	15	-1.039	4	0	1	0	5	0	4
47		5	max	196.664	2	1.028	6	-.041	12	0	5	.002	1	0	15
48			min	-218.684	3	.241	15	-.906	4	0	1	0	5	0	4
49		6	max	196.594	2	.851	6	-.041	12	0	5	.002	1	0	15
50			min	-218.736	3	.2	15	-.772	4	0	1	0	5	0	4
51		7	max	196.524	2	.675	6	-.041	12	0	5	.001	1	0	15
52			min	-218.789	3	.158	15	-.661	1	0	1	0	5	0	4
53		8	max	196.454	2	.499	6	-.041	12	0	5	.001	1	0	15
54			min	-218.841	3	.117	15	-.661	1	0	1	0	5	-.001	4
55		9	max	196.384	2	.322	6	-.041	12	0	5	.001	1	0	15
56			min	-218.894	3	.075	15	-.661	1	0	1	0	5	-.001	4
57		10	max	196.314	2	.146	6	-.041	12	0	5	0	1	0	15
58			min	-218.946	3	.034	15	-.661	1	0	1	0	5	-.001	4
59		11	max	196.244	2	.003	2	.046	5	0	5	0	1	0	15
60			min	-218.999	3	-.054	3	-.661	1	0	1	0	5	-.001	4
61		12	max	196.174	2	-.049	15	.18	5	0	5	0	1	0	15
62			min	-219.051	3	-.207	4	-.661	1	0	1	0	5	-.001	4
63		13	max	196.104	2	-.09	15	.314	5	0	5	0	1	0	15
64			min	-219.104	3	-.383	4	-.661	1	0	1	0	5	-.001	4
65		14	max	196.034	2	-.132	15	.447	5	0	5	0	1	0	15
66			min	-219.156	3	-.56	4	-.661	1	0	1	0	5	-.001	4
67		15	max	195.964	2	-.173	15	.581	5	0	5	0	1	0	15
68			min	-219.209	3	-.736	4	-.661	1	0	1	0	5	0	4
69		16	max	195.894	2	-.215	15	.715	5	0	5	0	1	0	15
70			min	-219.261	3	-.912	4	-.661	1	0	1	0	5	0	4
71		17	max	195.824	2	-.256	15	.848	5	0	5	0	10	0	15
72			min	-219.314	3	-1.089	4	-.661	1	0	1	0	5	0	4
73		18	max	195.754	2	-.298	15	.982	5	0	5	0	15	0	15
74			min	-219.366	3	-1.265	4	-.661	1	0	1	0	1	0	4
75		19	max	195.684	2	-.339	15	1.116	5	0	5	0	5	0	1
76			min	-219.419	3	-1.441	4	-.661	1	0	1	0	1	0	1
77	M4	1	max	424.383	1	0	1	-.238	12	0	1	0	5	0	1
78			min	26.101	15	0	1	-34.027	4	0	1	0	2	0	1
79		2	max	424.448	1	0	1	-.238	12	0	1	0	12	0	1
80			min	26.121	15	0	1	-34.083	4	0	1	-.003	4	0	1
81		3	max	424.512	1	0	1	-.238	12	0	1	0	12	0	1
82			min	26.14	15	0	1	-34.139	4	0	1	-.006	4	0	1
83		4	max	424.577	1	0	1	-.238	12	0	1	0	12	0	1
84			min	26.16	15	0	1	-34.195	4	0	1	-.009	4	0	1
85		5	max	424.642	1	0	1	-.238	12	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	26.179	15	0	1	-34.251	4	0	1	-.012	4	0	1
87	6	max	424.707	1	0	1	-.238	12	0	1	0	12	0	1
88		min	26.199	15	0	1	-34.308	4	0	1	-.015	4	0	1
89	7	max	424.771	1	0	1	-.238	12	0	1	0	12	0	1
90		min	26.218	15	0	1	-34.364	4	0	1	-.018	4	0	1
91	8	max	424.836	1	0	1	-.238	12	0	1	0	12	0	1
92		min	26.238	15	0	1	-34.42	4	0	1	-.021	4	0	1
93	9	max	424.901	1	0	1	-.238	12	0	1	0	12	0	1
94		min	26.257	15	0	1	-34.476	4	0	1	-.024	4	0	1
95	10	max	424.965	1	0	1	-.238	12	0	1	0	12	0	1
96		min	26.277	15	0	1	-34.532	4	0	1	-.028	4	0	1
97	11	max	425.03	1	0	1	-.238	12	0	1	0	12	0	1
98		min	26.296	15	0	1	-34.588	4	0	1	-.031	4	0	1
99	12	max	425.095	1	0	1	-.238	12	0	1	0	12	0	1
100		min	26.316	15	0	1	-34.644	4	0	1	-.034	4	0	1
101	13	max	425.16	1	0	1	-.238	12	0	1	0	12	0	1
102		min	26.335	15	0	1	-34.7	4	0	1	-.037	4	0	1
103	14	max	425.224	1	0	1	-.238	12	0	1	0	12	0	1
104		min	26.355	15	0	1	-34.756	4	0	1	-.04	4	0	1
105	15	max	425.289	1	0	1	-.238	12	0	1	0	12	0	1
106		min	26.374	15	0	1	-34.812	4	0	1	-.043	4	0	1
107	16	max	425.354	1	0	1	-.238	12	0	1	0	12	0	1
108		min	26.394	15	0	1	-34.868	4	0	1	-.046	4	0	1
109	17	max	425.418	1	0	1	-.238	12	0	1	0	12	0	1
110		min	26.413	15	0	1	-34.924	4	0	1	-.049	4	0	1
111	18	max	425.483	1	0	1	-.238	12	0	1	0	12	0	1
112		min	26.433	15	0	1	-34.98	4	0	1	-.052	4	0	1
113	19	max	425.548	1	0	1	-.238	12	0	1	0	12	0	1
114		min	26.452	15	0	1	-35.037	4	0	1	-.056	4	0	1
115	M6	1	max	899.369	1	.665	6	1.345	4	0	0	3	0	1
116		min	-1196.615	3	.148	15	-.137	3	0	5	0	11	0	1
117	2	max	899.504	1	.608	6	1.222	4	0	1	0	4	0	15
118		min	-1196.514	3	.134	15	-.137	3	0	5	0	11	0	6
119	3	max	899.639	1	.55	6	1.098	4	0	1	0	4	0	15
120		min	-1196.412	3	.121	15	-.137	3	0	5	0	10	0	6
121	4	max	899.774	1	.494	2	.975	4	0	1	0	4	0	15
122		min	-1196.311	3	.107	15	-.137	3	0	5	0	10	0	6
123	5	max	899.909	1	.449	2	.852	4	0	1	.001	4	0	15
124		min	-1196.21	3	.094	15	-.137	3	0	5	0	10	0	6
125	6	max	900.044	1	.404	2	.729	4	0	1	.001	4	0	15
126		min	-1196.109	3	.073	12	-.137	3	0	5	0	10	0	6
127	7	max	900.179	1	.36	2	.606	4	0	1	.001	4	0	15
128		min	-1196.008	3	.051	12	-.137	3	0	5	0	12	0	2
129	8	max	900.313	1	.315	2	.483	4	0	1	.001	4	0	15
130		min	-1195.907	3	.028	12	-.137	3	0	5	0	3	0	2
131	9	max	900.448	1	.27	2	.359	4	0	1	.001	4	0	15
132		min	-1195.806	3	0	3	-.137	3	0	5	0	3	0	2
133	10	max	900.583	1	.225	2	.236	4	0	1	.002	4	0	12
134		min	-1195.704	3	-.033	3	-.137	3	0	5	0	3	0	2
135	11	max	900.718	1	.18	2	.145	14	0	1	.002	4	0	12
136		min	-1195.603	3	-.066	3	-.137	3	0	5	0	3	0	2
137	12	max	900.853	1	.136	2	.138	1	0	1	.002	4	0	12
138		min	-1195.502	3	-.1	3	-.137	3	0	5	0	3	0	2
139	13	max	900.988	1	.091	2	.138	1	0	1	.002	4	0	12
140		min	-1195.401	3	-.133	3	-.19	5	0	5	0	3	0	2
141	14	max	901.123	1	.046	2	.138	1	0	1	.002	4	0	12
142		min	-1195.3	3	-.167	3	-.313	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	901.258	1	.001	2	.138	1	0	1	.001	4	0	12
144		min	-1195.199	3	-.201	3	-.436	5	0	5	0	3	0	2
145	16	max	901.392	1	-.043	2	.138	1	0	1	.001	4	0	12
146		min	-1195.097	3	-.234	3	-.559	5	0	5	0	3	0	2
147	17	max	901.527	1	-.068	15	.138	1	0	1	.001	4	0	3
148		min	-1194.996	3	-.268	3	-.682	5	0	5	0	3	0	2
149	18	max	901.662	1	-.082	15	.138	1	0	1	.001	4	0	3
150		min	-1194.895	3	-.313	4	-.806	5	0	5	0	3	0	2
151	19	max	901.797	1	-.095	15	.138	1	0	1	0	4	0	3
152		min	-1194.794	3	-.37	4	-.929	5	0	5	0	3	0	2
153	M7	1	max	789.336	2	1.756	.025	3	0	14	0	4	0	2
154		min	-688.454	3	.42	15	-1.283	5	0	3	0	3	0	3
155	2	max	789.266	2	1.58	4	.025	3	0	14	0	4	0	2
156		min	-688.507	3	.378	15	-1.149	5	0	3	0	3	0	3
157	3	max	789.196	2	1.403	4	.025	3	0	14	0	2	0	2
158		min	-688.559	3	.337	15	-1.015	5	0	3	0	3	0	3
159	4	max	789.126	2	1.227	4	.025	3	0	14	0	2	0	2
160		min	-688.612	3	.295	15	-.882	5	0	3	0	5	0	3
161	5	max	789.056	2	1.051	4	.025	3	0	14	0	2	0	15
162		min	-688.664	3	.254	15	-.748	5	0	3	0	5	0	3
163	6	max	788.986	2	.874	4	.025	3	0	14	0	2	0	15
164		min	-688.717	3	.212	15	-.614	5	0	3	0	5	0	3
165	7	max	788.916	2	.698	4	.025	3	0	14	0	2	0	15
166		min	-688.769	3	.171	15	-.481	5	0	3	0	5	0	6
167	8	max	788.846	2	.521	4	.025	3	0	14	0	2	0	15
168		min	-688.822	3	.118	12	-.347	5	0	3	0	5	-.001	6
169	9	max	788.776	2	.351	2	.025	3	0	14	0	2	0	15
170		min	-688.874	3	.049	12	-.213	5	0	3	0	5	-.001	6
171	10	max	788.706	2	.214	2	.025	3	0	14	0	2	0	15
172		min	-688.927	3	-.037	3	-.08	5	0	3	0	5	-.001	6
173	11	max	788.636	2	.076	2	.057	4	0	14	0	2	0	15
174		min	-688.979	3	-.14	3	-.004	10	0	3	0	5	-.001	6
175	12	max	788.566	2	-.037	15	.19	4	0	14	0	2	0	15
176		min	-689.032	3	-.243	3	-.004	10	0	3	0	5	-.001	6
177	13	max	788.496	2	-.078	15	.324	4	0	14	0	2	0	15
178		min	-689.084	3	-.361	6	-.004	10	0	3	0	5	-.001	6
179	14	max	788.426	2	-.119	15	.458	4	0	14	0	2	0	15
180		min	-689.137	3	-.537	6	-.004	10	0	3	0	5	-.001	6
181	15	max	788.356	2	-.161	15	.591	4	0	14	0	2	0	15
182		min	-689.189	3	-.714	6	-.004	10	0	3	0	5	0	6
183	16	max	788.286	2	-.202	15	.725	4	0	14	0	2	0	15
184		min	-689.242	3	-.89	6	-.004	10	0	3	0	5	0	6
185	17	max	788.216	2	-.244	15	.859	4	0	14	0	2	0	15
186		min	-689.294	3	-1.066	6	-.004	10	0	3	0	5	0	6
187	18	max	788.146	2	-.285	15	.992	4	0	14	0	2	0	15
188		min	-689.347	3	-1.243	6	-.004	10	0	3	0	3	0	6
189	19	max	788.076	2	-.327	15	1.126	4	0	14	0	4	0	1
190		min	-689.399	3	-1.419	6	-.004	10	0	3	0	3	0	1
191	M8	1	max	1048.743	1	0	.846	1	0	1	0	4	0	1
192		min	37.809	15	0	1	-33.882	4	0	1	0	1	0	1
193	2	max	1048.808	1	0	1	.846	1	0	1	0	1	0	1
194		min	37.829	15	0	1	-33.938	4	0	1	-.003	4	0	1
195	3	max	1048.872	1	0	1	.846	1	0	1	0	1	0	1
196		min	37.848	15	0	1	-33.994	4	0	1	-.006	4	0	1
197	4	max	1048.937	1	0	1	.846	1	0	1	0	1	0	1
198		min	37.868	15	0	1	-34.05	4	0	1	-.009	4	0	1
199	5	max	1049.002	1	0	1	.846	1	0	1	0	1	0	1





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	294.357	1	-.01	15	.005	3	.001	1	.002	4	0	15
258			min	-341.208	3	-.113	1	-.246	1	-.002	5	0	1	0	4
259		16	max	294.492	1	-.024	15	.005	3	.001	1	.002	5	0	15
260			min	-341.107	3	-.158	1	-.36	4	-.002	5	0	1	0	4
261		17	max	294.626	1	-.037	15	.005	3	.001	1	.001	5	0	15
262			min	-341.005	3	-.215	6	-.484	4	-.002	5	0	1	0	4
263		18	max	294.761	1	-.051	15	.005	3	.001	1	.001	5	0	12
264			min	-340.904	3	-.272	6	-.607	4	-.002	5	0	1	0	4
265		19	max	294.896	1	-.064	15	.005	3	.001	1	.001	5	0	12
266			min	-340.803	3	-.33	6	-.73	4	-.002	5	0	1	0	4
267	M11	1	max	196.674	2	1.721	6	.726	1	.002	4	0	5	0	1
268			min	-219.136	3	.398	15	-1.191	5	0	10	-.002	1	0	12
269		2	max	196.604	2	1.545	6	.726	1	.002	4	0	5	0	1
270			min	-219.188	3	.357	15	-1.057	5	0	10	-.002	1	0	3
271		3	max	196.534	2	1.368	6	.726	1	.002	4	0	5	0	1
272			min	-219.241	3	.315	15	-.924	5	0	10	-.002	1	0	3
273		4	max	196.464	2	1.192	6	.726	1	.002	4	0	5	0	15
274			min	-219.293	3	.274	15	-.79	5	0	10	-.002	1	0	4
275		5	max	196.394	2	1.015	6	.726	1	.002	4	0	3	0	15
276			min	-219.346	3	.232	15	-.656	5	0	10	-.002	1	0	4
277		6	max	196.324	2	.839	6	.726	1	.002	4	0	3	0	15
278			min	-219.398	3	.191	15	-.523	5	0	10	-.001	1	0	4
279		7	max	196.254	2	.663	6	.726	1	.002	4	0	3	0	15
280			min	-219.451	3	.15	15	-.389	5	0	10	-.001	1	-.001	4
281		8	max	196.184	2	.486	6	.726	1	.002	4	0	3	0	15
282			min	-219.503	3	.108	15	-.255	5	0	10	-.001	1	-.001	4
283		9	max	196.114	2	.31	6	.726	1	.002	4	0	3	0	15
284			min	-219.556	3	.067	15	-.122	5	0	10	0	1	-.001	4
285		10	max	196.044	2	.144	1	.726	1	.002	4	0	3	0	15
286			min	-219.608	3	.023	12	0	3	0	10	0	1	-.001	4
287		11	max	195.974	2	.006	1	.726	1	.002	4	0	3	0	15
288			min	-219.661	3	-.072	3	0	3	0	10	0	1	-.001	4
289		12	max	195.904	2	-.058	15	.726	1	.002	4	0	3	0	15
290			min	-219.713	3	-.22	4	0	3	0	10	0	1	-.001	4
291		13	max	195.834	2	-.099	15	.726	1	.002	4	0	3	0	15
292			min	-219.766	3	-.396	4	0	3	0	10	0	1	-.001	4
293		14	max	195.764	2	-.141	15	.726	1	.002	4	0	3	0	15
294			min	-219.818	3	-.573	4	0	3	0	10	0	1	-.001	4
295		15	max	195.694	2	-.182	15	.845	4	.002	4	0	3	0	15
296			min	-219.871	3	-.749	4	0	3	0	10	0	2	0	4
297		16	max	195.624	2	-.224	15	.978	4	.002	4	0	4	0	15
298			min	-219.923	3	-.925	4	0	3	0	10	0	10	0	4
299		17	max	195.554	2	-.265	15	1.112	4	.002	4	0	4	0	15
300			min	-219.976	3	-1.102	4	0	3	0	10	0	10	0	4
301		18	max	195.484	2	-.307	15	1.246	4	.002	4	0	4	0	15
302			min	-220.028	3	-1.278	4	0	3	0	10	0	10	0	4
303		19	max	195.414	2	-.348	15	1.379	4	.002	4	.001	4	0	1
304			min	-220.081	3	-1.454	4	0	3	0	10	0	10	0	1
305	M12	1	max	424.048	1	0	1	4.612	1	0	1	0	4	0	1
306			min	10.4	15	0	1	-30.97	5	0	1	0	3	0	1
307		2	max	424.113	1	0	1	4.612	1	0	1	0	1	0	1
308			min	10.419	15	0	1	-31.026	5	0	1	-.003	5	0	1
309		3	max	424.178	1	0	1	4.612	1	0	1	0	1	0	1
310			min	10.439	15	0	1	-31.082	5	0	1	-.005	5	0	1
311		4	max	424.242	1	0	1	4.612	1	0	1	.001	1	0	1
312			min	10.458	15	0	1	-31.138	5	0	1	-.008	5	0	1
313		5	max	424.307	1	0	1	4.612	1	0	1	.002	1	0	1



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 35° 85mph 30psf 7.75ft 7-05.rBabe 27





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-13.328	10	-28.514	2	-28.27	5	0	5	-.09	1	-.143	3
429	6	max	118.103	3	6.777	9	87.247	1	0	1	.059	5	.128	2
430		min	-13.194	10	-28.743	2	-28.028	5	0	5	-.071	1	-.141	3
431	7	max	118.223	3	6.587	9	87.247	1	0	1	.053	5	.134	2
432		min	-13.061	10	-28.972	2	-27.786	5	0	5	-.052	1	-.139	3
433	8	max	118.344	3	6.396	9	87.247	1	0	1	.047	5	.14	2
434		min	-12.928	10	-29.2	2	-27.544	5	0	5	-.033	1	-.137	3
435	9	max	118.464	3	6.206	9	87.247	1	0	1	.041	5	.146	2
436		min	-12.794	10	-29.429	2	-27.302	5	0	5	-.014	1	-.135	3
437	10	max	118.584	3	6.015	9	87.247	1	0	1	.035	4	.153	2
438		min	-12.661	10	-29.658	2	-27.06	5	0	5	0	2	-.133	3
439	11	max	118.704	3	5.824	9	87.247	1	0	1	.034	4	.159	2
440		min	-12.527	10	-29.886	2	-26.818	5	0	5	.002	10	-.13	3
441	12	max	118.824	3	5.634	9	87.247	1	0	1	.042	1	.166	2
442		min	-12.394	10	-30.115	2	-26.576	5	0	5	.005	10	-.128	3
443	13	max	118.944	3	5.443	9	87.247	1	0	1	.061	1	.172	2
444		min	-12.26	10	-30.344	2	-26.334	5	0	5	.006	12	-.126	3
445	14	max	119.064	3	5.252	9	87.247	1	0	1	.08	1	.179	2
446		min	-12.127	10	-30.573	2	-26.092	5	0	5	.007	12	-.123	3
447	15	max	119.184	3	5.062	9	87.247	1	0	1	.099	1	.186	2
448		min	-11.993	10	-30.801	2	-25.85	5	0	5	.004	15	-.121	3
449	16	max	95.194	2	141.372	2	87.83	1	0	10	.119	1	.191	2
450		min	4.486	15	-206.069	3	-24.401	5	0	4	.002	15	-.117	3
451	17	max	95.354	2	141.143	2	87.83	1	0	10	.138	1	.16	2
452		min	4.534	15	-206.241	3	-24.159	5	0	4	-.002	5	-.072	3
453	18	max	-1.383	15	370.911	2	92.609	1	0	2	.158	1	.081	2
454		min	-160.803	1	-165.678	3	-54.769	5	0	3	-.014	5	-.036	3
455	19	max	-1.334	15	370.682	2	92.609	1	0	2	.179	1	0	2
456		min	-160.643	1	-165.85	3	-54.527	5	0	3	-.026	5	0	3
457	M13	1	max	221.411	4	269.87	1	-5.379	15	0	.181	1	0	1
458		min	10.726	10	-342.669	3	-160.806	1	0	3	.004	15	0	3
459	2	max	212.953	4	190.299	1	-3.69	15	0	2	.059	1	.252	3
460		min	10.726	10	-241.572	3	-123.401	1	0	3	0	15	-.198	1
461	3	max	204.496	4	110.728	1	-2.002	15	0	2	.002	3	.416	3
462		min	10.726	10	-140.475	3	-85.997	1	0	3	-.031	1	-.328	1
463	4	max	196.038	4	31.158	1	-.313	15	0	2	-.002	12	.493	3
464		min	10.726	10	-39.377	3	-48.593	1	0	3	-.089	1	-.389	1
465	5	max	187.581	4	61.72	3	2.021	5	0	2	-.003	15	.484	3
466		min	10.726	10	-48.413	1	-11.188	1	0	3	-.115	1	-.381	1
467	6	max	179.123	4	162.817	3	26.216	1	0	2	0	15	.387	3
468		min	10.726	10	-127.983	1	.435	12	0	3	-.109	1	-.305	1
469	7	max	170.666	4	263.914	3	63.621	1	0	2	.004	5	.203	3
470		min	10.726	10	-207.554	1	2.073	12	0	3	-.07	1	-.161	1
471	8	max	162.208	4	365.011	3	101.025	1	0	2	.011	5	.052	1
472		min	10.726	10	-287.125	1	3.711	12	0	3	0	3	-.067	3
473	9	max	153.751	4	466.109	3	138.429	1	0	2	.104	1	.334	1
474		min	10.726	10	-366.695	1	5.349	12	0	3	.004	12	-.425	3
475	10	max	145.293	4	567.206	3	175.834	1	0	2	.239	1	.684	1
476		min	10.726	10	-446.266	1	6.987	12	0	3	.009	12	-.87	3
477	11	max	109.628	4	366.695	1	-1.686	15	0	3	.1	1	.334	1
478		min	4.944	12	-466.109	3	-137.688	1	0	2	-.013	5	-.425	3
479	12	max	101.171	4	287.125	1	.003	15	0	3	.001	2	.052	1
480		min	4.944	12	-365.011	3	-100.284	1	0	2	-.015	4	-.067	3
481	13	max	92.713	4	207.554	1	2.351	5	0	3	-.005	12	.203	3
482		min	4.944	12	-263.914	3	-62.88	1	0	2	-.073	1	-.161	1
483	14	max	92.462	1	127.983	1	4.963	5	0	3	-.006	12	.387	3
484		min	4.944	12	-162.817	3	-25.475	1	0	2	-.111	1	-.305	1





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-54.907	3	0	2	-.03	1	0	3	0	3	-.003	1
543		6	max	0	2	.94	1	.036	3	0	1	0	1	0	2
544			min	-54.982	3	0	2	-.03	1	0	3	0	3	-.004	1
545		7	max	0	2	.705	1	.036	3	0	1	0	3	0	2
546			min	-55.058	3	0	2	-.03	1	0	3	0	1	-.004	1
547		8	max	0	2	.47	1	.036	3	0	1	0	3	0	2
548			min	-55.133	3	0	2	-.03	1	0	3	0	1	-.004	1
549		9	max	0	2	.235	1	.036	3	0	1	0	3	0	2
550			min	-55.209	3	0	2	-.03	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.036	3	0	1	0	3	0	2
552			min	-55.284	3	0	1	-.03	1	0	3	0	1	-.005	1
553		11	max	0	2	0	2	.036	3	0	1	0	3	0	2
554			min	-55.36	3	-.235	1	-.03	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.036	3	0	1	0	3	0	2
556			min	-55.435	3	-.47	1	-.03	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.036	3	0	1	0	3	0	2
558			min	-55.511	3	-.705	1	-.03	1	0	3	0	1	-.004	1
559		14	max	0	2	0	2	.036	3	0	1	0	3	0	2
560			min	-55.586	3	-.94	1	-.03	1	0	3	0	1	-.004	1
561		15	max	0	2	0	2	.036	3	0	1	0	3	0	2
562			min	-55.662	3	-1.174	1	-.03	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.036	3	0	1	0	3	0	2
564			min	-55.738	3	-1.409	1	-.03	1	0	3	0	1	-.003	1
565		17	max	0	2	0	2	.036	3	0	1	0	3	0	2
566			min	-55.813	3	-1.644	1	-.03	1	0	3	0	1	-.002	1
567		18	max	0	2	0	2	.036	3	0	1	0	3	0	2
568			min	-55.889	3	-1.879	1	-.03	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.036	3	0	1	0	3	0	1
570			min	-55.964	3	-2.114	1	-.03	1	0	3	0	1	0	1
571	M16A	1	max	-1.073	10	3.491	4	.313	4	0	3	0	3	0	1
572			min	-256.212	4	1.057	12	-.014	3	0	2	0	4	0	1
573		2	max	-.989	10	3.103	4	.281	4	0	3	0	3	0	12
574			min	-256.284	4	.94	12	-.014	3	0	2	0	4	-.002	4
575		3	max	-.905	10	2.715	4	.25	4	0	3	0	3	0	12
576			min	-256.356	4	.822	12	-.014	3	0	2	0	4	-.003	4
577		4	max	-.822	10	2.327	4	.218	4	0	3	0	3	-.001	12
578			min	-256.429	4	.705	12	-.014	3	0	2	0	4	-.004	4
579		5	max	-.738	10	1.939	4	.187	4	0	3	0	3	-.002	12
580			min	-256.501	4	.587	12	-.014	3	0	2	0	1	-.005	4
581		6	max	-.654	10	1.551	4	.155	4	0	3	0	5	-.002	12
582			min	-256.573	4	.47	12	-.014	3	0	2	0	1	-.006	4
583		7	max	-.57	10	1.164	4	.124	4	0	3	0	5	-.002	12
584			min	-256.646	4	.352	12	-.014	3	0	2	0	1	-.007	4
585		8	max	-.486	10	.776	4	.092	4	0	3	0	5	-.002	12
586			min	-256.718	4	.235	12	-.014	3	0	2	0	1	-.007	4
587		9	max	-.402	10	.388	4	.06	4	0	3	0	5	-.002	12
588			min	-256.79	4	.117	12	-.014	3	0	2	0	1	-.007	4
589		10	max	-.318	10	0	1	.029	4	0	3	0	5	-.002	12
590			min	-256.862	4	0	1	-.014	3	0	2	0	1	-.007	4
591		11	max	-.234	10	-.117	12	.02	1	0	3	0	5	-.002	12
592			min	-256.935	4	-.388	4	-.014	3	0	2	0	1	-.007	4
593		12	max	-.15	10	-.235	12	.02	1	0	3	0	5	-.002	12
594			min	-257.007	4	-.776	4	-.038	5	0	2	0	1	-.007	4
595		13	max	-.066	10	-.352	12	.02	1	0	3	0	5	-.002	12
596			min	-257.079	4	-1.164	4	-.069	5	0	2	0	3	-.007	4
597		14	max	.018	10	-.47	12	.02	1	0	3	0	4	-.002	12
598			min	-257.151	4	-1.551	4	-.101	5	0	2	0	3	-.006	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.102	10	-5.587	12	.02	1	0	3	0	4	-.002	12
600		min	-257.224	4	-1.939	4	-.133	5	0	2	0	3	-.005	4
601	16	max	.186	10	-7.705	12	.02	1	0	3	0	4	-.001	12
602		min	-257.296	4	-2.327	4	-.164	5	0	2	0	3	-.004	4
603	17	max	.269	10	-8.822	12	.02	1	0	3	0	1	0	12
604		min	-257.368	4	-2.715	4	-.196	5	0	2	0	5	-.003	4
605	18	max	.353	10	-.94	12	.02	1	0	3	0	1	0	12
606		min	-257.44	4	-3.103	4	-.227	5	0	2	0	5	-.002	4
607	19	max	.437	10	-1.057	12	.02	1	0	3	0	1	0	1
608		min	-257.513	4	-3.491	4	-.259	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.011	2	.017	1	2.018e-3	5	NC	3	NC	3	
2			min	-.004	3	-.011	3	-.019	5	-1.541e-3	1	4010	2	2451.642	1	
3			2	max	.003	1	.01	.016	1	2.041e-3	5	NC	3	NC	3	
4				min	-.004	3	-.011	3	-.018	5	-1.473e-3	1	4387.587	2	2629.789	1
5			3	max	.003	1	.009	2	.015	1	2.064e-3	5	NC	1	NC	3
6				min	-.003	3	-.01	3	-.018	5	-1.404e-3	1	4838.714	2	2840.938	1
7			4	max	.002	1	.008	2	.014	1	2.087e-3	5	NC	1	NC	3
8				min	-.003	3	-.01	3	-.018	5	-1.336e-3	1	5381.435	2	3092.805	1
9			5	max	.002	1	.007	2	.013	1	2.11e-3	5	NC	1	NC	3
10				min	-.003	3	-.009	3	-.017	5	-1.268e-3	1	6039.899	2	3395.693	1
11			6	max	.002	1	.006	2	.011	1	2.133e-3	5	NC	1	NC	3
12				min	-.003	3	-.009	3	-.016	5	-1.2e-3	1	6846.93	2	3763.597	1
13			7	max	.002	1	.005	2	.01	1	2.156e-3	5	NC	1	NC	3
14				min	-.003	3	-.008	3	-.016	5	-1.131e-3	1	7847.996	2	4215.911	1
15			8	max	.002	1	.005	2	.009	1	2.179e-3	5	NC	1	NC	2
16				min	-.002	3	-.008	3	-.015	5	-1.063e-3	1	9107.469	2	4780.142	1
17			9	max	.002	1	.004	2	.008	1	2.202e-3	5	NC	1	NC	2
18				min	-.002	3	-.007	3	-.014	5	-9.948e-4	1	NC	1	5496.384	1
19			10	max	.001	1	.003	2	.007	1	2.225e-3	5	NC	1	NC	2
20				min	-.002	3	-.007	3	-.013	5	-9.265e-4	1	NC	1	6425.023	1
21		11	max	.001	1	.003	2	.006	1	2.248e-3	5	NC	1	NC	2	
22			min	-.002	3	-.006	3	-.012	5	-8.582e-4	1	NC	1	7660.615	1	
23		12	max	.001	1	.002	2	.005	1	2.271e-3	5	NC	1	NC	2	
24			min	-.001	3	-.005	3	-.011	5	-7.9e-4	1	NC	1	9358.492	1	
25		13	max	0	1	.002	2	.004	1	2.294e-3	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.01	5	-7.217e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.003	1	2.317e-3	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.008	5	-6.535e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.34e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.007	5	-5.852e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	2.363e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.005	5	-5.169e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.386e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.004	5	-4.487e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.409e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.002	5	-3.804e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.432e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.122e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.496e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.163e-3	5	NC	1	NC	1	
41			2	max	0	3	0	2	.006	5	1.819e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-1.183e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.012	5	2.141e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	-.001	1	-1.203e-3	5	NC	1	8437.033	14
45		4	max	0	3	0	2	.018	5	2.464e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	-.001	1	-1.222e-3	5	NC	1	5512.245	14
47		5	max	0	3	0	2	.024	5	2.787e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	-1.242e-3	5	NC	1	4061.769	14
49		6	max	0	3	0	2	.029	5	3.11e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.262e-3	5	NC	1	3199.754	14
51		7	max	0	3	0	2	.035	4	3.433e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.282e-3	5	NC	1	2631.087	14
53		8	max	0	3	.001	2	.041	4	3.756e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.301e-3	5	NC	1	2229.407	14
55		9	max	.001	3	.001	2	.047	4	4.079e-4	1	NC	1	NC	1
56			min	0	2	-.007	3	0	2	-1.321e-3	5	NC	1	1931.609	14
57		10	max	.001	3	.002	2	.053	4	4.402e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-1.341e-3	5	NC	1	1702.693	14
59		11	max	.001	3	.002	2	.059	4	4.725e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-1.36e-3	5	NC	1	1521.697	14
61		12	max	.002	3	.003	2	.064	4	5.048e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	12	-1.38e-3	5	NC	1	1375.308	14
63		13	max	.002	3	.004	2	.07	4	5.371e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	12	-1.4e-3	5	NC	1	1254.673	14
65		14	max	.002	3	.004	2	.076	4	5.694e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	12	-1.42e-3	5	NC	1	1153.677	14
67		15	max	.002	3	.005	2	.081	4	6.016e-4	1	NC	1	NC	2
68			min	-.002	2	-.009	3	0	12	-1.439e-3	5	8562.915	2	1067.963	14
69		16	max	.002	3	.006	2	.087	4	6.339e-4	1	NC	1	NC	2
70			min	-.002	2	-.009	3	0	12	-1.459e-3	5	7263.734	2	994.341	14
71		17	max	.002	3	.007	2	.092	4	6.662e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	12	-1.479e-3	5	6257.501	2	930.425	14
73		18	max	.002	3	.008	2	.097	4	6.985e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	12	-1.498e-3	5	5469.185	2	874.395	14
75		19	max	.002	3	.01	2	.103	4	7.308e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	12	-1.518e-3	5	4846.022	2	824.835	14
77	M4	1	max	.002	1	.012	2	0	12	8.328e-3	5	NC	1	NC	3
78			min	0	15	-.011	3	-.109	4	-1.167e-3	1	NC	1	178.045	4
79		2	max	.002	1	.012	2	0	12	8.328e-3	5	NC	1	NC	3
80			min	0	15	-.01	3	-.1	4	-1.167e-3	1	NC	1	194.095	4
81		3	max	.002	1	.011	2	0	12	8.328e-3	5	NC	1	NC	3
82			min	0	15	-.01	3	-.091	4	-1.167e-3	1	NC	1	213.2	4
83		4	max	.002	1	.01	2	0	12	8.328e-3	5	NC	1	NC	3
84			min	0	15	-.009	3	-.082	4	-1.167e-3	1	NC	1	236.163	4
85		5	max	.002	1	.01	2	0	12	8.328e-3	5	NC	1	NC	2
86			min	0	15	-.008	3	-.073	4	-1.167e-3	1	NC	1	264.08	4
87		6	max	.001	1	.009	2	0	12	8.328e-3	5	NC	1	NC	2
88			min	0	15	-.008	3	-.065	4	-1.167e-3	1	NC	1	298.475	4
89		7	max	.001	1	.008	2	0	12	8.328e-3	5	NC	1	NC	2
90			min	0	15	-.007	3	-.057	4	-1.167e-3	1	NC	1	341.52	4
91		8	max	.001	1	.008	2	0	12	8.328e-3	5	NC	1	NC	2
92			min	0	15	-.007	3	-.049	4	-1.167e-3	1	NC	1	396.392	4
93		9	max	.001	1	.007	2	0	12	8.328e-3	5	NC	1	NC	2
94			min	0	15	-.006	3	-.041	4	-1.167e-3	1	NC	1	467.899	4
95		10	max	.001	1	.006	2	0	12	8.328e-3	5	NC	1	NC	2
96			min	0	15	-.005	3	-.034	4	-1.167e-3	1	NC	1	563.598	4
97		11	max	0	1	.006	2	0	12	8.328e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.028	4	-1.167e-3	1	NC	1	695.935	4
99		12	max	0	1	.005	2	0	12	8.328e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.022	4	-1.167e-3	1	NC	1	886.623	4
101		max	0	1	.004	2	0	12	8.328e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.016	4	-1.167e-3	1	NC	1	1176.385	4
103		max	0	1	.003	2	0	12	8.328e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.012	4	-1.167e-3	1	NC	1	1649.226	4
105		max	0	1	.003	2	0	12	8.328e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.008	4	-1.167e-3	1	NC	1	2502.506	4
107		max	0	1	.002	2	0	12	8.328e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.004	4	-1.167e-3	1	NC	1	4297.278	4
109		max	0	1	.001	2	0	12	8.328e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.002	4	-1.167e-3	1	NC	1	9212.566	4
111		max	0	1	0	2	0	12	8.328e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-1.167e-3	1	NC	1	NC	1
113		max	0	1	0	1	0	1	8.328e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-1.167e-3	1	NC	1	NC	1
115	M6	max	.009	1	.039	2	.005	1	2.231e-3	4	NC	3	NC	2
116		min	-.013	3	-.036	3	-.019	5	1.512e-6	10	1099.747	2	8079.769	1
117		max	.009	1	.036	2	.005	1	2.249e-3	4	NC	3	NC	2
118		min	-.012	3	-.034	3	-.019	5	6.146e-7	10	1176.997	2	8784.209	1
119		max	.008	1	.034	2	.004	1	2.267e-3	4	NC	3	NC	2
120		min	-.011	3	-.032	3	-.018	5	-2.83e-7	10	1265.535	2	9617.218	1
121		max	.008	1	.031	2	.004	1	2.285e-3	4	NC	3	NC	1
122		min	-.01	3	-.03	3	-.018	5	-3.24e-6	2	1367.606	2	NC	1
123		max	.007	1	.029	2	.004	1	2.302e-3	4	NC	3	NC	1
124		min	-.01	3	-.028	3	-.018	5	-6.585e-6	2	1486.104	2	NC	1
125		max	.007	1	.026	2	.003	1	2.32e-3	4	NC	3	NC	1
126		min	-.009	3	-.026	3	-.017	5	-9.931e-6	2	1624.814	2	NC	1
127		max	.006	1	.024	2	.003	1	2.338e-3	4	NC	3	NC	1
128		min	-.008	3	-.024	3	-.017	5	-1.328e-5	2	1788.782	2	NC	1
129		max	.006	1	.021	2	.002	1	2.356e-3	4	NC	3	NC	1
130		min	-.008	3	-.022	3	-.016	5	-1.662e-5	2	1984.889	2	9558.806	4
131		max	.005	1	.019	2	.002	1	2.373e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.015	5	-1.997e-5	2	2222.765	2	9054.551	4
133		max	.005	1	.017	2	.002	1	2.391e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.014	5	-2.331e-5	2	2516.308	2	8829.063	4
135		max	.004	1	.015	2	.001	1	2.409e-3	4	NC	3	NC	1
136		min	-.006	3	-.016	3	-.013	5	-2.666e-5	2	2886.355	2	8852.816	4
137		max	.004	1	.013	2	.001	1	2.427e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.012	5	-3.e-5	2	3365.627	2	9135.737	4
139		max	.003	1	.011	2	0	1	2.444e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.01	5	-3.335e-5	2	4008.626	2	9732.624	4
141		max	.003	1	.009	2	0	1	2.462e-3	4	NC	3	NC	1
142		min	-.003	3	-.01	3	-.009	5	-3.67e-5	2	4913.41	2	NC	1
143		max	.002	1	.007	2	0	1	2.48e-3	4	NC	3	NC	1
144		min	-.003	3	-.008	3	-.007	5	-4.004e-5	2	6276.019	2	NC	1
145		max	.002	1	.005	2	0	1	2.498e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.006	5	-4.339e-5	2	8553.766	2	NC	1
147		max	.001	1	.003	2	0	1	2.515e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.004	5	-4.673e-5	2	NC	1	NC	1
149		max	0	1	.002	2	0	1	2.534e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-5.008e-5	2	NC	1	NC	1
151		max	0	1	0	1	0	1	2.554e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.342e-5	2	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.536e-5	2	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.222e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.006	5	2.114e-5	2	NC	1	NC	1
156		min	0	2	-.002	3	0	2	-1.227e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.012	5	2.034e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	2	-1.234e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.018	5	2.038e-5	1	NC	1	NC	1
160			min	-.001	2	-.007	3	0	2	-1.241e-3	4	9612.452	2	NC	1
161		5	max	.002	3	.006	2	.025	5	2.041e-5	1	NC	1	NC	1
162			min	-.002	2	-.009	3	0	2	-1.247e-3	4	7252.888	2	NC	1
163		6	max	.002	3	.008	2	.031	5	2.418e-5	3	NC	3	NC	1
164			min	-.002	2	-.011	3	0	1	-1.254e-3	4	5809.967	2	NC	1
165		7	max	.003	3	.01	2	.037	5	4.183e-5	3	NC	3	NC	1
166			min	-.003	2	-.013	3	0	1	-1.261e-3	4	4827.446	2	NC	1
167		8	max	.003	3	.011	2	.043	5	5.948e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-1.267e-3	4	4110.229	2	NC	1
169		9	max	.003	3	.013	2	.048	4	7.714e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-1.274e-3	4	3561.002	2	NC	1
171		10	max	.004	3	.015	2	.054	4	9.479e-5	3	NC	3	NC	1
172			min	-.004	2	-.018	3	-.001	1	-1.281e-3	4	3125.786	2	NC	1
173		11	max	.004	3	.017	2	.06	4	1.124e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.287e-3	4	2772.176	2	NC	1
175		12	max	.005	3	.019	2	.065	4	1.301e-4	3	NC	3	NC	1
176			min	-.005	2	-.021	3	-.001	1	-1.294e-3	4	2479.472	2	NC	1
177		13	max	.005	3	.021	2	.071	4	1.477e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.301e-3	4	2233.76	2	NC	1
179		14	max	.006	3	.023	2	.076	4	1.654e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-1.308e-3	4	2025.282	2	NC	1
181		15	max	.006	3	.025	2	.082	4	1.831e-4	3	NC	3	NC	1
182			min	-.007	2	-.025	3	-.002	1	-1.314e-3	4	1846.946	2	NC	1
183		16	max	.007	3	.027	2	.087	4	2.007e-4	3	NC	3	NC	1
184			min	-.007	2	-.026	3	-.002	1	-1.321e-3	4	1693.443	2	NC	1
185		17	max	.007	3	.03	2	.092	4	2.184e-4	3	NC	3	NC	1
186			min	-.008	2	-.027	3	-.002	1	-1.328e-3	4	1560.698	2	NC	1
187		18	max	.007	3	.032	2	.097	4	2.36e-4	3	NC	3	NC	1
188			min	-.008	2	-.028	3	-.002	1	-1.334e-3	4	1445.519	2	NC	1
189		19	max	.008	3	.034	2	.102	4	2.537e-4	3	NC	3	NC	1
190			min	-.009	2	-.029	3	-.003	1	-1.341e-3	4	1345.369	2	NC	1
191	M8	1	max	.005	1	.045	2	.003	1	8.165e-3	4	NC	1	NC	2
192			min	0	15	-.035	3	-.108	4	-2.128e-4	1	NC	1	178.838	4
193		2	max	.005	1	.042	2	.002	1	8.165e-3	4	NC	1	NC	2
194			min	0	15	-.033	3	-.099	4	-2.128e-4	1	NC	1	194.958	4
195		3	max	.004	1	.04	2	.002	1	8.165e-3	4	NC	1	NC	2
196			min	0	15	-.031	3	-.09	4	-2.128e-4	1	NC	1	214.146	4
197		4	max	.004	1	.037	2	.002	1	8.165e-3	4	NC	1	NC	2
198			min	0	15	-.029	3	-.081	4	-2.128e-4	1	NC	1	237.21	4
199		5	max	.004	1	.035	2	.002	1	8.165e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.073	4	-2.128e-4	1	NC	1	265.249	4
201		6	max	.004	1	.032	2	.002	1	8.165e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.064	4	-2.128e-4	1	NC	1	299.795	4
203		7	max	.003	1	.03	2	.001	1	8.165e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.056	4	-2.128e-4	1	NC	1	343.029	4
205		8	max	.003	1	.027	2	.001	1	8.165e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.049	4	-2.128e-4	1	NC	1	398.141	4
207		9	max	.003	1	.025	2	.001	1	8.165e-3	4	NC	1	NC	1
208			min	0	15	-.02	3	-.041	4	-2.128e-4	1	NC	1	469.961	4
209		10	max	.002	1	.022	2	0	1	8.165e-3	4	NC	1	NC	1
210			min	0	15	-.018	3	-.034	4	-2.128e-4	1	NC	1	566.078	4
211		11	max	.002	1	.02	2	0	1	8.165e-3	4	NC	1	NC	1
212			min	0	15	-.016	3	-.028	4	-2.128e-4	1	NC	1	698.995	4
213		12	max	.002	1	.017	2	0	1	8.165e-3	4	NC	1	NC	1



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 35° 85mph 30psf 7.75ft 7-05.rdb Page 36



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.01	4	2.322e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.21e-3	4	NC	1	NC	1
273		4	max	0	3	0	2	.015	4	3.551e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.324e-3	4	NC	1	NC	1
275		5	max	0	3	0	2	.02	4	-1.092e-5	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-1.439e-3	4	NC	1	NC	1
277		6	max	0	3	0	2	.025	5	-2.34e-5	12	NC	1	NC	1
278			min	0	2	-.005	3	-.002	1	-1.554e-3	4	NC	1	NC	1
279		7	max	0	3	0	2	.03	5	-3.587e-5	12	NC	1	NC	1
280			min	0	2	-.005	3	-.002	1	-1.669e-3	4	NC	1	NC	1
281		8	max	0	3	0	2	.035	5	-4.835e-5	12	NC	1	NC	1
282			min	0	2	-.006	3	-.003	1	-1.784e-3	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.04	5	-5.449e-5	10	NC	1	NC	1
284			min	0	2	-.007	3	-.004	1	-1.898e-3	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.045	5	-6.032e-5	10	NC	1	NC	2
286			min	-.001	2	-.007	3	-.006	1	-2.013e-3	4	NC	1	8203.085	1
287		11	max	.001	3	.002	2	.05	5	-6.614e-5	10	NC	1	NC	2
288			min	-.001	2	-.008	3	-.007	1	-2.128e-3	4	NC	1	6714.762	1
289		12	max	.002	3	.003	2	.055	5	-7.197e-5	10	NC	1	NC	2
290			min	-.001	2	-.008	3	-.008	1	-2.243e-3	4	NC	1	5637.325	1
291		13	max	.002	3	.004	2	.06	5	-7.779e-5	10	NC	1	NC	2
292			min	-.001	2	-.008	3	-.01	1	-2.358e-3	4	NC	1	4831.982	1
293		14	max	.002	3	.004	2	.065	5	-8.362e-5	10	NC	1	NC	2
294			min	-.002	2	-.009	3	-.011	1	-2.472e-3	4	NC	1	4214.427	1
295		15	max	.002	3	.005	2	.069	5	-8.944e-5	10	NC	1	NC	3
296			min	-.002	2	-.009	3	-.012	1	-2.587e-3	4	8573.683	2	3730.996	1
297		16	max	.002	3	.006	2	.074	5	-9.527e-5	10	NC	1	NC	3
298			min	-.002	2	-.009	3	-.014	1	-2.702e-3	4	7272.13	2	3346.189	1
299		17	max	.002	3	.007	2	.079	5	-1.011e-4	10	NC	1	NC	3
300			min	-.002	2	-.009	3	-.015	1	-2.817e-3	4	6264.216	2	3035.767	1
301		18	max	.002	3	.008	2	.084	5	-1.069e-4	10	NC	1	NC	3
302			min	-.002	2	-.009	3	-.017	1	-2.931e-3	4	5474.687	2	2782.728	1
303		19	max	.002	3	.009	2	.089	5	-1.127e-4	10	NC	3	NC	3
304			min	-.002	2	-.009	3	-.018	1	-3.046e-3	4	4850.633	2	2574.888	1
305	M12	1	max	.002	1	.012	2	.015	1	9.8e-3	4	NC	1	NC	3
306			min	0	15	-.011	3	-.099	5	1.182e-4	10	NC	1	195.509	5
307		2	max	.002	1	.012	2	.013	1	9.8e-3	4	NC	1	NC	3
308			min	0	15	-.01	3	-.091	5	1.182e-4	10	NC	1	213.128	5
309		3	max	.002	1	.011	2	.012	1	9.8e-3	4	NC	1	NC	3
310			min	0	15	-.01	3	-.083	5	1.182e-4	10	NC	1	234.099	5
311		4	max	.002	1	.01	2	.011	1	9.8e-3	4	NC	1	NC	3
312			min	0	15	-.009	3	-.075	5	1.182e-4	10	NC	1	259.306	5
313		5	max	.002	1	.01	2	.01	1	9.8e-3	4	NC	1	NC	3
314			min	0	15	-.009	3	-.067	5	1.182e-4	10	NC	1	289.952	5
315		6	max	.001	1	.009	2	.009	1	9.8e-3	4	NC	1	NC	3
316			min	0	15	-.008	3	-.059	5	1.182e-4	10	NC	1	327.708	5
317		7	max	.001	1	.008	2	.008	1	9.8e-3	4	NC	1	NC	3
318			min	0	15	-.007	3	-.052	5	1.182e-4	10	NC	1	374.958	5
319		8	max	.001	1	.008	2	.007	1	9.8e-3	4	NC	1	NC	3
320			min	0	15	-.007	3	-.044	5	1.182e-4	10	NC	1	435.19	5
321		9	max	.001	1	.007	2	.006	1	9.8e-3	4	NC	1	NC	3
322			min	0	15	-.006	3	-.038	5	1.182e-4	10	NC	1	513.681	5
323		10	max	.001	1	.006	2	.005	1	9.8e-3	4	NC	1	NC	2
324			min	0	15	-.005	3	-.031	5	1.182e-4	10	NC	1	618.725	5
325		11	max	0	1	.006	2	.004	1	9.8e-3	4	NC	1	NC	2
326			min	0	15	-.005	3	-.025	5	1.182e-4	10	NC	1	763.983	5
327		12	max	0	1	.005	2	.003	1	9.8e-3	4	NC	1	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.02	5	1.182e-4	10	NC	1	973.286	5
329		max	0	1	.004	2	.002	1	9.8e-3	4	NC	1	NC	2
330		min	0	15	-.004	3	-.015	5	1.182e-4	10	NC	1	1291.331	5
331		max	0	1	.003	2	.002	1	9.8e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.011	5	1.182e-4	10	NC	1	1810.317	5
333		max	0	1	.003	2	.001	1	9.8e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.007	5	1.182e-4	10	NC	1	2746.852	5
335		max	0	1	.002	2	0	1	9.8e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.004	5	1.182e-4	10	NC	1	4716.709	5
337		max	0	1	.001	2	0	1	9.8e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.002	5	1.182e-4	10	NC	1	NC	1
339		max	0	1	0	2	0	1	9.8e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	1.182e-4	10	NC	1	NC	1
341		max	0	1	0	1	0	1	9.8e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.182e-4	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.01	5	2.208e-2	1	NC	1	NC	1
344		min	-.009	2	-.024	2	-.006	1	-2.791e-2	3	NC	1	NC	1
345		max	.01	3	.016	3	.014	5	1.048e-2	1	NC	4	NC	2
346		min	-.009	2	-.015	2	-.013	1	-1.383e-2	3	4780.778	2	6584.853	1
347		max	.01	3	.007	3	.019	5	7.459e-4	5	NC	4	NC	2
348		min	-.009	2	-.005	2	-.017	1	-9.079e-4	1	2454.309	2	3994.039	1
349		max	.009	3	.003	1	.024	5	7.7e-4	5	NC	4	NC	3
350		min	-.009	2	-.002	3	-.02	1	-7.843e-4	1	1717.451	2	3306.194	1
351		max	.009	3	.01	2	.03	5	7.942e-4	5	NC	4	NC	3
352		min	-.009	2	-.008	3	-.02	1	-6.608e-4	1	1361.525	2	2400.165	5
353		max	.009	3	.016	2	.035	5	8.183e-4	5	NC	5	NC	3
354		min	-.009	2	-.014	3	-.019	1	-5.372e-4	1	1158.958	2	1844.508	5
355		max	.009	3	.021	2	.042	5	8.424e-4	5	NC	5	NC	2
356		min	-.009	2	-.018	3	-.017	1	-4.137e-4	1	1034.992	2	1484.621	5
357		max	.009	3	.025	2	.048	5	8.666e-4	5	NC	5	NC	2
358		min	-.009	2	-.021	3	-.014	1	-2.902e-4	1	958.511	2	1235.12	5
359		max	.009	3	.027	2	.055	5	8.907e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.01	1	-1.666e-4	1	915.006	2	1051.293	4
361		max	.009	3	.028	2	.061	5	9.148e-4	5	NC	5	NC	1
362		min	-.009	2	-.024	3	-.005	1	-4.308e-5	1	897.862	2	895.944	4
363		max	.009	3	.027	2	.068	4	9.574e-4	4	NC	5	NC	1
364		min	-.009	2	-.023	3	-.001	1	1.446e-5	10	905.278	2	780.014	4
365		max	.009	3	.026	2	.076	4	1.011e-3	4	NC	5	NC	2
366		min	-.009	2	-.021	3	0	10	2.835e-5	10	939.628	2	691.494	4
367		max	.009	3	.022	2	.083	4	1.066e-3	4	NC	5	NC	2
368		min	-.009	2	-.018	3	0	12	3.403e-5	12	1008.74	2	622.753	4
369		max	.009	3	.017	2	.09	4	1.12e-3	4	NC	5	NC	3
370		min	-.009	2	-.014	3	0	12	3.916e-5	12	1130.452	2	568.756	4
371		max	.009	3	.01	2	.097	4	1.174e-3	4	NC	4	NC	3
372		min	-.009	2	-.008	3	0	12	4.429e-5	12	1346.341	2	526.089	4
373		max	.009	3	.002	1	.103	4	1.617e-3	4	NC	4	NC	3
374		min	-.009	2	-.002	3	0	12	4.788e-5	12	1756.301	1	492.387	4
375		max	.009	3	.006	3	.108	4	1.132e-2	4	NC	4	NC	2
376		min	-.009	2	-.008	2	0	12	-2.03e-4	1	2469.426	1	466.014	4
377		max	.009	3	.014	3	.113	4	1.5e-2	2	NC	2	NC	2
378		min	-.009	2	-.02	2	0	10	-6.835e-3	3	4772.44	1	445.767	4
379		max	.009	3	.023	3	.117	4	3.039e-2	2	NC	1	NC	1
380		min	-.009	2	-.032	2	-.004	1	-1.382e-2	3	5850.789	2	431.349	4
381	M5	max	.031	3	.088	3	.009	5	8.937e-6	4	NC	1	NC	1
382		min	-.035	2	-.083	2	-.007	1	5.472e-8	10	3474.446	3	NC	1
383		max	.031	3	.054	3	.014	5	3.808e-4	5	NC	5	NC	1
384		min	-.035	2	-.05	2	-.006	1	-5.222e-5	1	1418.769	2	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.022	3	.019	5	7.466e-4	5	NC	5	NC	1
386		min	-.035	2	-.018	2	-.005	1	-1.045e-4	1	727.894	2	NC	1
387	4	max	.03	3	.01	2	.025	5	7.788e-4	5	NC	5	NC	1
388		min	-.035	2	-.005	3	-.005	1	-9.986e-5	1	508.784	2	NC	1
389	5	max	.03	3	.034	2	.031	5	8.11e-4	5	NC	5	NC	1
390		min	-.035	2	-.028	3	-.004	1	-9.526e-5	1	402.897	2	NC	1
391	6	max	.03	3	.054	2	.037	5	8.432e-4	5	NC	15	NC	1
392		min	-.035	2	-.046	3	-.004	1	-9.066e-5	1	342.599	2	NC	1
393	7	max	.03	3	.071	2	.044	5	8.754e-4	5	NC	15	NC	1
394		min	-.035	2	-.06	3	-.003	1	-8.606e-5	1	305.663	2	NC	1
395	8	max	.03	3	.083	2	.051	5	9.076e-4	5	NC	15	NC	1
396		min	-.035	2	-.07	3	-.003	1	-8.146e-5	1	282.836	2	NC	1
397	9	max	.03	3	.091	2	.057	5	9.398e-4	5	NC	15	NC	1
398		min	-.035	2	-.075	3	-.003	1	-7.686e-5	1	269.8	2	NC	1
399	10	max	.03	3	.095	2	.064	5	9.721e-4	5	NC	15	NC	1
400		min	-.035	2	-.077	3	-.002	1	-7.226e-5	1	264.584	2	NC	1
401	11	max	.03	3	.093	2	.071	5	1.004e-3	5	NC	15	NC	1
402		min	-.035	2	-.074	3	-.002	1	-6.766e-5	1	266.648	2	NC	1
403	12	max	.03	3	.087	2	.078	5	1.036e-3	5	NC	15	NC	1
404		min	-.035	2	-.068	3	-.002	1	-6.306e-5	1	276.693	2	NC	1
405	13	max	.029	3	.075	2	.085	5	1.069e-3	5	NC	15	NC	1
406		min	-.034	2	-.058	3	-.002	1	-5.845e-5	1	297.045	2	NC	1
407	14	max	.029	3	.058	2	.091	5	1.101e-3	5	NC	15	NC	1
408		min	-.034	2	-.044	3	-.002	1	-5.385e-5	1	333.021	2	NC	1
409	15	max	.029	3	.035	2	.097	4	1.133e-3	5	NC	5	NC	1
410		min	-.034	2	-.027	3	-.002	1	-4.925e-5	1	397.068	2	NC	1
411	16	max	.029	3	.007	1	.103	4	1.554e-3	5	NC	5	NC	1
412		min	-.034	2	-.006	3	-.003	1	-5.418e-5	1	523.931	2	NC	1
413	17	max	.029	3	.019	3	.108	4	1.13e-2	4	NC	5	NC	1
414		min	-.035	2	-.028	2	-.003	1	-2.859e-4	1	848.461	1	NC	1
415	18	max	.029	3	.046	3	.113	4	5.793e-3	4	NC	5	NC	1
416		min	-.034	2	-.068	2	-.003	1	-1.465e-4	1	1669.064	3	NC	1
417	19	max	.029	3	.074	3	.117	4	2.004e-6	5	NC	3	NC	1
418		min	-.034	2	-.111	2	-.003	1	-3.37e-7	3	1669.669	2	NC	1
419	M9	1	max	.01	.027	3	.008	5	2.791e-2	3	NC	1	NC	1
420		min	-.009	2	-.024	2	-.008	1	-2.208e-2	1	NC	1	NC	1
421	2	max	.01	3	.016	3	.008	5	1.38e-2	3	NC	4	NC	2
422		min	-.009	2	-.015	2	-.002	1	-1.076e-2	1	4781.965	2	7630.805	1
423	3	max	.01	3	.006	3	.008	4	3.566e-4	1	NC	4	NC	2
424		min	-.009	2	-.005	2	0	3	-4.872e-5	3	2454.934	2	4739.502	1
425	4	max	.01	3	.003	1	.011	4	2.522e-4	1	NC	4	NC	3
426		min	-.009	2	-.002	3	0	3	-5.495e-5	3	1717.886	2	4018.775	1
427	5	max	.009	3	.01	2	.014	4	1.478e-4	1	NC	4	NC	3
428		min	-.009	2	-.009	3	-.002	3	-6.119e-5	3	1361.852	2	3987.548	1
429	6	max	.009	3	.016	2	.018	4	1.347e-4	4	NC	4	NC	3
430		min	-.009	2	-.014	3	-.002	3	-6.743e-5	3	1159.214	2	3719.729	14
431	7	max	.009	3	.021	2	.022	4	1.489e-4	5	NC	5	NC	2
432		min	-.009	2	-.018	3	-.003	3	-7.367e-5	3	1035.196	2	2761.252	4
433	8	max	.009	3	.025	2	.027	4	1.847e-4	5	NC	5	NC	2
434		min	-.009	2	-.021	3	-.003	3	-1.653e-4	1	958.672	2	2129.879	4
435	9	max	.009	3	.027	2	.033	5	2.204e-4	5	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	1	-2.697e-4	1	915.13	2	1696.396	4
437	10	max	.009	3	.028	2	.04	5	2.562e-4	5	NC	5	NC	1
438		min	-.009	2	-.024	3	-.008	1	-3.741e-4	1	897.948	2	1385.85	4
439	11	max	.009	3	.027	2	.047	5	2.92e-4	5	NC	5	NC	2
440		min	-.009	2	-.023	3	-.011	1	-4.784e-4	1	905.321	2	1155.656	4
441	12	max	.009	3	.026	2	.055	5	3.278e-4	5	NC	5	NC	2





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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.274	3	.152	1	7.27e-3	2	NC	15	NC	3
500		min	-.117	4	-.583	2	.011	10	-4.94e-3	3	337.581	2	1188.278	1
501	4	max	.002	1	.346	3	.23	1	8.534e-3	2	NC	15	NC	12
502		min	-.117	4	-.738	2	.019	10	-5.762e-3	3	263.559	2	794.523	1
503	5	max	.002	1	.368	3	.268	1	9.799e-3	2	NC	15	8910.866	12
504		min	-.117	4	-.782	2	.022	10	-6.584e-3	3	248.049	2	682.173	1
505	6	max	.003	1	.341	3	.256	1	1.106e-2	2	NC	15	8955.256	15
506		min	-.117	4	-.718	2	.019	10	-7.406e-3	3	271.336	2	712.777	1
507	7	max	.003	1	.275	3	.197	1	1.233e-2	2	NC	5	NC	5
508		min	-.117	4	-.567	2	.01	10	-8.227e-3	3	347.938	2	921.351	1
509	8	max	.003	1	.189	3	.108	1	1.359e-2	2	NC	5	NC	5
510		min	-.117	4	-.371	2	-.003	10	-9.049e-3	3	549.017	2	1653.762	1
511	9	max	.003	1	.11	3	.033	3	1.486e-2	2	NC	5	NC	2
512		min	-.117	4	-.192	2	-.017	2	-9.871e-3	3	1165.414	2	7754.225	3
513	10	max	.003	1	.074	3	.029	3	1.612e-2	2	NC	4	NC	1
514		min	-.117	4	-.111	2	-.034	2	-1.069e-2	3	2376.333	2	7434.622	2
515	11	max	.003	1	.11	3	.029	3	1.486e-2	2	NC	5	NC	2
516		min	-.117	4	-.192	2	-.016	2	-9.87e-3	3	1165.414	2	7987.729	14
517	12	max	.003	1	.189	3	.105	1	1.359e-2	2	NC	5	NC	5
518		min	-.117	4	-.371	2	-.003	10	-9.047e-3	3	549.017	2	1694.382	1
519	13	max	.003	1	.275	3	.193	1	1.233e-2	2	NC	5	NC	10
520		min	-.117	4	-.567	2	.01	10	-8.224e-3	3	347.938	2	939.273	1
521	14	max	.003	1	.341	3	.252	1	1.106e-2	2	NC	15	NC	5
522		min	-.117	4	-.718	2	.01	15	-7.402e-3	3	271.336	2	725.873	1
523	15	max	.004	1	.368	3	.263	1	9.8e-3	2	NC	15	NC	5
524		min	-.117	4	-.782	2	.005	15	-6.579e-3	3	248.049	2	695.27	1
525	16	max	.004	1	.346	3	.225	1	8.536e-3	2	NC	15	NC	5
526		min	-.117	4	-.738	2	-.002	5	-5.756e-3	3	263.559	2	811.892	1
527	17	max	.004	1	.274	3	.148	1	7.271e-3	2	NC	15	NC	3
528		min	-.117	4	-.583	2	-.009	5	-4.934e-3	3	337.581	2	1220.999	1
529	18	max	.004	1	.161	3	.058	1	6.007e-3	2	NC	5	NC	3
530		min	-.117	4	-.336	2	-.01	5	-4.111e-3	3	613.329	2	2964.032	1
531	19	max	.004	1	.023	3	.009	3	4.742e-3	2	NC	1	NC	1
532		min	-.117	4	-.032	2	-.009	2	-3.288e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.992e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.467e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.015	4	9.369e-4	3	NC	5	NC	1
536		min	0	5	-.018	1	0	3	-7.775e-4	5	5587.996	2	7008.803	4
537	3	max	0	3	-.001	15	.033	4	1.475e-3	3	NC	5	NC	1
538		min	-.002	5	-.036	1	-.004	3	-1.2e-3	2	2843.541	2	3104.669	4
539	4	max	0	3	-.002	15	.053	4	2.012e-3	3	NC	5	NC	9
540		min	-.003	5	-.053	1	-.008	3	-1.767e-3	2	1950.835	2	1942.553	4
541	5	max	0	3	-.003	15	.071	4	2.55e-3	3	NC	5	NC	9
542		min	-.004	5	-.067	1	-.013	3	-2.334e-3	2	1522.256	2	1426.807	4
543	6	max	0	3	-.003	15	.088	4	3.088e-3	3	NC	5	NC	9
544		min	-.005	5	-.08	1	-.019	3	-2.901e-3	2	1281.14	2	1157.369	4
545	7	max	0	3	-.003	15	.101	4	3.626e-3	3	NC	5	8045.915	9
546		min	-.006	5	-.09	1	-.026	3	-3.468e-3	2	1136.14	2	1008.518	4
547	8	max	0	3	-.003	15	.11	4	4.163e-3	3	NC	5	6710.278	9
548		min	-.007	5	-.098	1	-.031	3	-4.035e-3	2	1049.118	2	930.839	4
549	9	max	0	3	-.003	15	.113	4	4.701e-3	3	NC	5	5828.651	9
550		min	-.008	5	-.102	1	-.037	3	-4.602e-3	2	1002.278	2	903.581	4
551	10	max	0	3	-.003	15	.111	4	5.239e-3	3	NC	15	5245.075	9
552		min	-.009	5	-.104	1	-.041	3	-5.169e-3	2	987.46	2	920.132	4
553	11	max	0	3	-.003	15	.103	4	5.776e-3	3	NC	5	4877.186	9
554		min	-.01	5	-.103	1	-.044	3	-5.736e-3	2	1002.278	2	984.076	4
555	12	max	0	3	-.002	15	.092	4	6.314e-3	3	NC	5	4684.831	9





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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.