



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

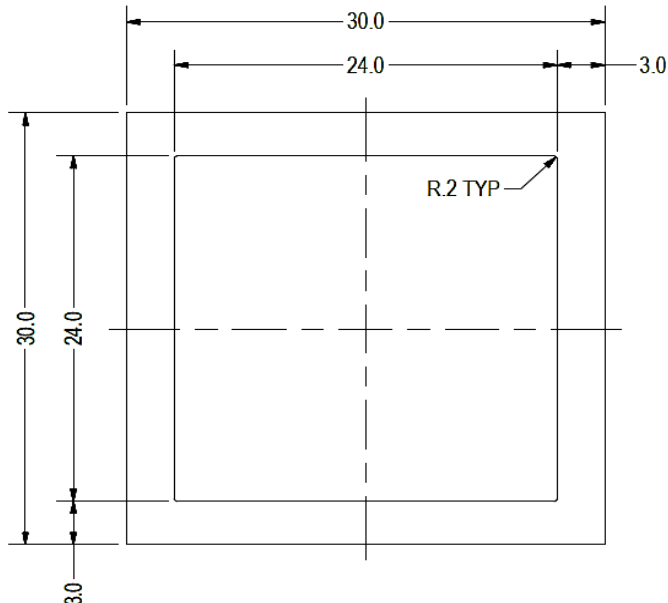
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.627 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>5%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.718 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>19%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

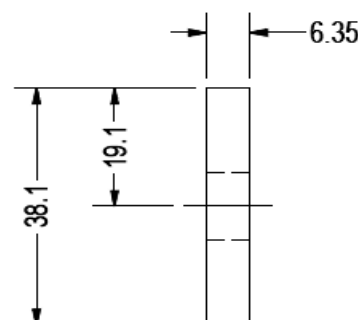
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.588 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<b>13%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.137 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>6%</b>



A cross brace kit is required every 36 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

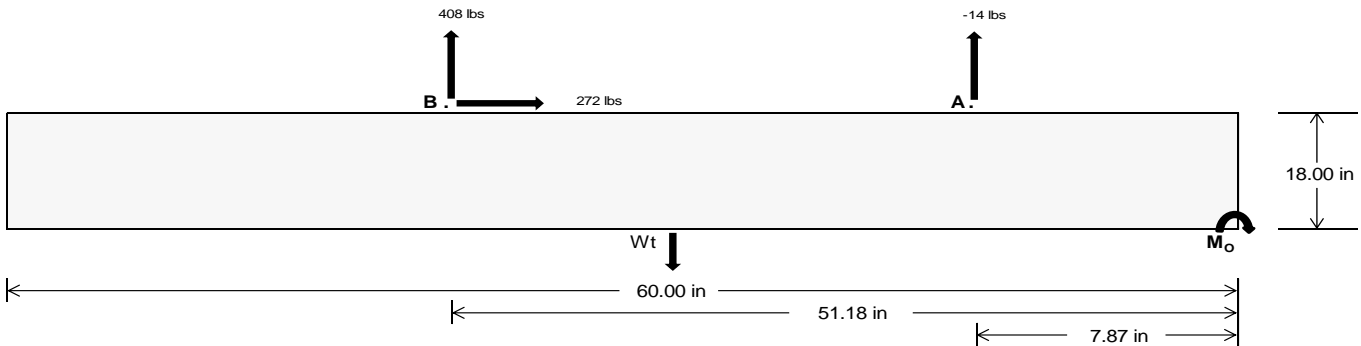
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>18.86</b>	<b>1770.57</b>	k
Compressive Load =	<b>814.94</b>	<b>1152.77</b>	k
Lateral Load =	<b>1.54</b>	<b>1178.27</b>	k
Moment (Weak Axis) =	<b>0.00</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 25663.8$  in-lbs  
Resisting Force Required = 855.46 lbs  
S.F. = 1.67  
Weight Required = 1425.77 lbs  
Minimum Width = 20 in  
Weight Provided = 1812.50 lbs

### Sliding

Force = 271.81 lbs  
Friction = 0.4  
Weight Required = 679.53 lbs  
Resisting Weight = 1812.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 271.81 lbs  
Cohesion = 130 psf  
Area = 8.33 ft<sup>2</sup>  
Resisting = 906.25 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

	Ballast Width			
	20 in	21 in	22 in	23 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$	1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
$F_A$	275 lbs	275 lbs	275 lbs	275 lbs	309 lbs	309 lbs	309 lbs	309 lbs	409 lbs	409 lbs	409 lbs	409 lbs	28 lbs	28 lbs	28 lbs	28 lbs
$F_B$	175 lbs	175 lbs	175 lbs	175 lbs	501 lbs	501 lbs	501 lbs	501 lbs	488 lbs	488 lbs	488 lbs	488 lbs	-816 lbs	-816 lbs	-816 lbs	-816 lbs
$F_V$	25 lbs	25 lbs	25 lbs	25 lbs	489 lbs	489 lbs	489 lbs	489 lbs	383 lbs	383 lbs	383 lbs	383 lbs	-544 lbs	-544 lbs	-544 lbs	-544 lbs
$P_{total}$	2262 lbs	2353 lbs	2444 lbs	2534 lbs	2623 lbs	2713 lbs	2804 lbs	2895 lbs	2710 lbs	2801 lbs	2892 lbs	2982 lbs	299 lbs	354 lbs	408 lbs	462 lbs
$M$	236 lbs-ft	236 lbs-ft	236 lbs-ft	236 lbs-ft	418 lbs-ft	418 lbs-ft	418 lbs-ft	418 lbs-ft	468 lbs-ft	468 lbs-ft	468 lbs-ft	468 lbs-ft	676 lbs-ft	676 lbs-ft	676 lbs-ft	676 lbs-ft
$e$	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	2.26 ft	1.91 ft	1.66 ft	1.46 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	237.6 psf	236.6 psf	235.7 psf	234.9 psf	254.6 psf	252.8 psf	251.2 psf	249.7 psf	257.9 psf	256.0 psf	254.2 psf	252.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	305.4 psf	301.2 psf	297.4 psf	294.0 psf	374.9 psf	367.4 psf	360.6 psf	354.4 psf	392.6 psf	384.2 psf	376.7 psf	369.7 psf	495.3 psf	228.7 psf	175.9 psf	154.9 psf

Maximum Bearing Pressure = 495 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

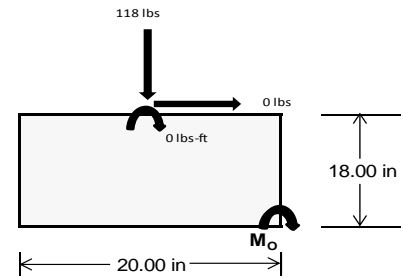
#### Overturning Check

$M_o = 0.0 \text{ ft-lbs}$   
 Resisting Force Required = 0.00 lbs  
 S.F. = 1.67  
 Weight Required = 0.00 lbs  
 Minimum Width = 20 in  
 Weight Provided = 1812.50 lbs

*A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	51 lbs	118 lbs	48 lbs	142 lbs	378 lbs	139 lbs	15 lbs	34 lbs	14 lbs
$F_v$	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2295 lbs	2362 lbs	2292 lbs	2278 lbs	2514 lbs	2275 lbs	671 lbs	691 lbs	670 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
$f_{min}$	275.3 sqft	283.4 sqft	275.0 sqft	273.0 sqft	301.6 sqft	272.8 sqft	80.5 sqft	82.9 sqft	80.4 sqft
$f_{max}$	275.5 psf	283.4 psf	275.1 psf	273.7 psf	301.9 psf	273.2 psf	80.5 psf	82.9 psf	80.4 psf



Maximum Bearing Pressure = 302 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.799 k
Allowable Uplift =	1.214 k
Utilization =	<u>66%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.075 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.627 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>

#### Diagonal Strut

Maximum Axial Load =	0.718 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



#### Rear Strut

Maximum Axial Load =	1.056 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.137 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	33.11 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.662 in
Max Drift, $\Delta_{MAX}$ =	0.005 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$117.177$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$121.682$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.271 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.31 \\
 &21.5027 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.31 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-40.249	-40.249	0	0
2	M16	Y	-40.249	-40.249	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-118.221	-118.221	0	0
2	M16	y	-197.035	-197.035	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	236.442	236.442	0	0
2	M16	y	118.221	118.221	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								
8																				
9	ASD 1.0D + 1.0S	Yes	Y		1	1	3	1												
10	ASD 1.0D + 0.6W	Yes	Y		1	1			4	.6										
11	ASD 1.0D + 0.75L + 0.45W + 0....	Yes	Y		1	1	3	.75	4	.45										
12	ASD 0.6D + 0.6W	Yes	Y		2	.6					5	.6								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y		1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y		1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y		1	.362					6	.875								



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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	267.133	2	289.677	2	.004	10	0	10	0	1	0	1
2		min	-309.336	3	-439.161	3	-.147	3	0	3	0	1	0	1
3	N7	max	.025	3	230.653	1	.035	10	0	10	0	1	0	1
4		min	-.122	2	9.376	15	-.524	1	0	1	0	1	0	1
5	N15	max	.126	3	626.878	1	.12	9	0	9	0	1	0	1
6		min	-1.181	2	20.532	15	-.791	3	-.001	3	0	1	0	1
7	N16	max	823.333	2	886.745	2	0	2	0	9	0	1	0	1
8		min	-906.358	3	-1361.98	3	-97.859	3	0	3	0	1	0	1
9	N23	max	.026	3	231.052	1	.577	1	0	1	0	1	0	1
10		min	-.122	2	9.491	15	-.034	10	0	10	0	1	0	1
11	N24	max	267.134	2	292.031	2	98.789	3	0	9	0	1	0	1
12		min	-310.181	3	-438.798	3	-.004	10	0	3	0	1	0	1
13	Totals:	max	1356.176	2	2448.816	2	0	11						
14		min	-1525.698	3	-2074.191	3	0	3						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	190.068	2	.679	4	.077	1	0	10	0	10	0	1
2			min	-360.769	3	.16	15	-.06	3	0	3	0	1	0	1
3		2	max	190.203	2	.621	4	.077	1	0	10	0	10	0	15
4			min	-360.667	3	.146	15	-.06	3	0	3	0	3	0	4
5		3	max	190.338	2	.564	4	.077	1	0	10	0	10	0	15
6			min	-360.566	3	.133	15	-.06	3	0	3	0	3	0	4
7		4	max	190.473	2	.506	4	.077	1	0	10	0	15	0	15
8			min	-360.465	3	.119	15	-.06	3	0	3	0	3	0	4
9		5	max	190.608	2	.449	4	.077	1	0	10	0	9	0	15
10			min	-360.364	3	.106	15	-.06	3	0	3	0	3	0	4
11		6	max	190.743	2	.391	4	.077	1	0	10	0	9	0	15
12			min	-360.263	3	.092	15	-.06	3	0	3	0	3	0	4
13		7	max	190.878	2	.334	4	.077	1	0	10	0	9	0	15
14			min	-360.162	3	.079	15	-.06	3	0	3	0	3	0	4
15		8	max	191.012	2	.276	4	.077	1	0	10	0	9	0	15
16			min	-360.061	3	.065	15	-.06	3	0	3	0	3	0	4
17		9	max	191.147	2	.219	4	.077	1	0	10	0	9	0	15
18			min	-359.959	3	.051	15	-.06	3	0	3	0	3	0	4
19		10	max	191.282	2	.161	4	.077	1	0	10	0	9	0	15
20			min	-359.858	3	.038	15	-.06	3	0	3	0	3	0	4
21		11	max	191.417	2	.111	2	.077	1	0	10	0	9	0	15
22			min	-359.757	3	.016	12	-.06	3	0	3	0	3	0	4
23		12	max	191.552	2	.066	2	.077	1	0	10	0	9	0	15
24			min	-359.656	3	-.013	3	-.06	3	0	3	0	3	0	4
25		13	max	191.687	2	.021	2	.077	1	0	10	0	9	0	15
26			min	-359.555	3	-.047	3	-.06	3	0	3	0	3	0	4
27		14	max	191.822	2	-.016	15	.077	1	0	10	0	9	0	15
28			min	-359.454	3	-.08	3	-.06	3	0	3	0	3	0	4
29		15	max	191.956	2	-.03	15	.077	1	0	10	0	9	0	15
30			min	-359.353	3	-.126	4	-.06	3	0	3	0	3	0	4
31		16	max	192.091	2	-.043	15	.077	1	0	10	0	9	0	15
32			min	-359.251	3	-.184	4	-.06	3	0	3	0	3	0	4
33		17	max	192.226	2	-.057	15	.077	1	0	10	0	9	0	15
34			min	-359.15	3	-.241	4	-.06	3	0	3	0	3	0	4
35		18	max	192.361	2	-.07	15	.077	1	0	10	0	9	0	15
36			min	-359.049	3	-.299	4	-.06	3	0	3	0	3	0	4
37		19	max	192.496	2	-.084	15	.077	1	0	10	0	9	0	15

***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-358.948	3	-.356	4	-.06	3	0	3	0	3	0	4
39	M3	1	max	232.738	2	1.736	4	.006	10	0	10	0	1	0	4
40			min	-219.294	3	.408	15	-.114	1	0	1	0	10	0	15
41		2	max	232.668	2	1.56	4	.006	10	0	10	0	1	0	2
42			min	-219.347	3	.367	15	-.114	1	0	1	0	10	0	3
43		3	max	232.598	2	1.383	4	.006	10	0	10	0	1	0	2
44			min	-219.399	3	.325	15	-.114	1	0	1	0	10	0	3
45		4	max	232.528	2	1.207	4	.006	10	0	10	0	1	0	15
46			min	-219.452	3	.284	15	-.114	1	0	1	0	10	0	4
47		5	max	232.458	2	1.03	4	.006	10	0	10	0	1	0	15
48			min	-219.504	3	.242	15	-.114	1	0	1	0	10	0	4
49		6	max	232.388	2	.854	4	.006	10	0	10	0	1	0	15
50			min	-219.557	3	.201	15	-.114	1	0	1	0	10	0	4
51		7	max	232.318	2	.678	4	.006	10	0	10	0	1	0	15
52			min	-219.609	3	.159	15	-.114	1	0	1	0	10	0	4
53		8	max	232.248	2	.501	4	.006	10	0	10	0	1	0	15
54			min	-219.662	3	.118	15	-.114	1	0	1	0	10	-.001	4
55		9	max	232.178	2	.325	4	.006	10	0	10	0	1	0	15
56			min	-219.714	3	.076	15	-.114	1	0	1	0	10	-.001	4
57		10	max	232.108	2	.149	4	.006	10	0	10	0	1	0	15
58			min	-219.767	3	.035	15	-.114	1	0	1	0	10	-.001	4
59		11	max	232.038	2	.006	2	.006	10	0	10	0	1	0	15
60			min	-219.819	3	-.053	3	-.114	1	0	1	0	10	-.001	4
61		12	max	231.968	2	-.048	15	.006	10	0	10	0	1	0	15
62			min	-219.872	3	-.204	4	-.114	1	0	1	0	10	-.001	4
63		13	max	231.898	2	-.089	15	.006	10	0	10	0	1	0	15
64			min	-219.924	3	-.381	4	-.114	1	0	1	0	10	-.001	4
65		14	max	231.828	2	-.131	15	.006	10	0	10	0	1	0	15
66			min	-219.977	3	-.557	4	-.114	1	0	1	0	10	-.001	4
67		15	max	231.758	2	-.172	15	.006	10	0	10	0	1	0	15
68			min	-220.029	3	-.733	4	-.114	1	0	1	0	10	0	4
69		16	max	231.688	2	-.214	15	.006	10	0	10	0	9	0	15
70			min	-220.082	3	-.91	4	-.114	1	0	1	0	10	0	4
71		17	max	231.618	2	-.255	15	.006	10	0	10	0	10	0	15
72			min	-220.134	3	-1.086	4	-.114	1	0	1	0	1	0	4
73		18	max	231.548	2	-.297	15	.006	10	0	10	0	10	0	15
74			min	-220.187	3	-1.262	4	-.114	1	0	1	0	1	0	4
75		19	max	231.478	2	-.338	15	.006	10	0	10	0	10	0	1
76			min	-220.239	3	-1.439	4	-.114	1	0	1	0	1	0	1
77	M4	1	max	229.488	1	0	1	.035	10	0	1	0	3	0	1
78			min	9.024	15	0	1	-.545	1	0	1	0	2	0	1
79		2	max	229.553	1	0	1	.035	10	0	1	0	15	0	1
80			min	9.044	15	0	1	-.545	1	0	1	0	1	0	1
81		3	max	229.618	1	0	1	.035	10	0	1	0	10	0	1
82			min	9.063	15	0	1	-.545	1	0	1	0	1	0	1
83		4	max	229.682	1	0	1	.035	10	0	1	0	10	0	1
84			min	9.083	15	0	1	-.545	1	0	1	0	1	0	1
85		5	max	229.747	1	0	1	.035	10	0	1	0	10	0	1
86			min	9.102	15	0	1	-.545	1	0	1	0	1	0	1
87		6	max	229.812	1	0	1	.035	10	0	1	0	10	0	1
88			min	9.122	15	0	1	-.545	1	0	1	0	1	0	1
89		7	max	229.876	1	0	1	.035	10	0	1	0	10	0	1
90			min	9.141	15	0	1	-.545	1	0	1	0	1	0	1
91		8	max	229.941	1	0	1	.035	10	0	1	0	10	0	1
92			min	9.161	15	0	1	-.545	1	0	1	0	1	0	1
93		9	max	230.006	1	0	1	.035	10	0	1	0	10	0	1
94			min	9.18	15	0	1	-.545	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	230.071	1	0	1	.035	10	0	1	0	10	0	1
96		min	9.2	15	0	1	-.545	1	0	1	0	1	0	1
97	11	max	230.135	1	0	1	.035	10	0	1	0	10	0	1
98		min	9.219	15	0	1	-.545	1	0	1	0	1	0	1
99	12	max	230.2	1	0	1	.035	10	0	1	0	10	0	1
100		min	9.239	15	0	1	-.545	1	0	1	0	1	0	1
101	13	max	230.265	1	0	1	.035	10	0	1	0	10	0	1
102		min	9.259	15	0	1	-.545	1	0	1	0	1	0	1
103	14	max	230.329	1	0	1	.035	10	0	1	0	10	0	1
104		min	9.278	15	0	1	-.545	1	0	1	0	1	0	1
105	15	max	230.394	1	0	1	.035	10	0	1	0	10	0	1
106		min	9.298	15	0	1	-.545	1	0	1	0	1	0	1
107	16	max	230.459	1	0	1	.035	10	0	1	0	10	0	1
108		min	9.317	15	0	1	-.545	1	0	1	0	1	0	1
109	17	max	230.523	1	0	1	.035	10	0	1	0	10	0	1
110		min	9.337	15	0	1	-.545	1	0	1	0	1	0	1
111	18	max	230.588	1	0	1	.035	10	0	1	0	10	0	1
112		min	9.356	15	0	1	-.545	1	0	1	0	1	0	1
113	19	max	230.653	1	0	1	.035	10	0	1	0	10	0	1
114		min	9.376	15	0	1	-.545	1	0	1	0	1	0	1
115	M6	1	max	585.242	2	.679	.014	9	0	3	0	3	0	1
116		min	-1056.077	3	.16	15	-.281	3	0	2	0	2	0	1
117	2	max	585.376	2	.622	4	.014	9	0	3	0	3	0	15
118		min	-1055.976	3	.146	15	-.281	3	0	2	0	2	0	4
119	3	max	585.511	2	.564	4	.014	9	0	3	0	3	0	15
120		min	-1055.875	3	.133	15	-.281	3	0	2	0	2	0	4
121	4	max	585.646	2	.507	4	.014	9	0	3	0	3	0	15
122		min	-1055.774	3	.119	15	-.281	3	0	2	0	2	0	4
123	5	max	585.781	2	.449	4	.014	9	0	3	0	3	0	15
124		min	-1055.673	3	.102	12	-.281	3	0	2	0	2	0	4
125	6	max	585.916	2	.395	2	.014	9	0	3	0	3	0	15
126		min	-1055.571	3	.08	12	-.281	3	0	2	0	1	0	4
127	7	max	586.051	2	.35	2	.014	9	0	3	0	3	0	15
128		min	-1055.47	3	.058	12	-.281	3	0	2	0	1	0	4
129	8	max	586.186	2	.305	2	.014	9	0	3	0	9	0	15
130		min	-1055.369	3	.035	12	-.281	3	0	2	0	3	0	4
131	9	max	586.32	2	.26	2	.014	9	0	3	0	9	0	12
132		min	-1055.268	3	.007	3	-.281	3	0	2	0	3	0	4
133	10	max	586.455	2	.216	2	.014	9	0	3	0	9	0	12
134		min	-1055.167	3	-.026	3	-.281	3	0	2	0	3	0	4
135	11	max	586.59	2	.171	2	.014	9	0	3	0	9	0	12
136		min	-1055.066	3	-.06	3	-.281	3	0	2	0	3	0	2
137	12	max	586.725	2	.126	2	.014	9	0	3	0	9	0	12
138		min	-1054.965	3	-.093	3	-.281	3	0	2	0	3	0	2
139	13	max	586.86	2	.081	2	.014	9	0	3	0	9	0	12
140		min	-1054.863	3	-.127	3	-.281	3	0	2	0	3	0	2
141	14	max	586.995	2	.036	2	.014	9	0	3	0	9	0	12
142		min	-1054.762	3	-.161	3	-.281	3	0	2	0	3	0	2
143	15	max	587.13	2	-.008	2	.014	9	0	3	0	9	0	12
144		min	-1054.661	3	-.194	3	-.281	3	0	2	0	3	0	2
145	16	max	587.265	2	-.043	15	.014	9	0	3	0	9	0	12
146		min	-1054.56	3	-.228	3	-.281	3	0	2	0	3	0	2
147	17	max	587.399	2	-.057	15	.014	9	0	3	0	9	0	3
148		min	-1054.459	3	-.261	3	-.281	3	0	2	0	3	0	2
149	18	max	587.534	2	-.07	15	.014	9	0	3	0	9	0	3
150		min	-1054.358	3	-.298	4	-.281	3	0	2	0	3	0	2
151	19	max	587.669	2	-.084	15	.014	9	0	3	0	9	0	3





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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1054.256	3	-.355	4	-.281	3	0	2	0	3	0	2
153	M7	1	max	717.751	2	1.739	4	.054	3	0	9	0	1	0
154		min	-609.928	3	.409	15	-.014	1	0	3	0	3	0	3
155		2	max	717.681	2	1.562	4	.054	3	0	9	0	1	0
156		min	-609.98	3	.367	15	-.014	1	0	3	0	3	0	3
157		3	max	717.611	2	1.386	4	.054	3	0	9	0	1	0
158		min	-610.033	3	.326	15	-.014	1	0	3	0	3	0	3
159		4	max	717.541	2	1.21	4	.054	3	0	9	0	1	0
160		min	-610.085	3	.284	15	-.014	1	0	3	0	3	0	3
161		5	max	717.471	2	1.033	4	.054	3	0	9	0	1	0
162		min	-610.138	3	.243	15	-.014	1	0	3	0	3	0	3
163		6	max	717.401	2	.857	4	.054	3	0	9	0	1	0
164		min	-610.19	3	.201	15	-.014	1	0	3	0	3	0	4
165		7	max	717.331	2	.68	4	.054	3	0	9	0	1	0
166		min	-610.243	3	.16	15	-.014	1	0	3	0	3	0	4
167		8	max	717.261	2	.504	4	.054	3	0	9	0	1	0
168		min	-610.295	3	.118	15	-.014	1	0	3	0	3	-.001	4
169		9	max	717.191	2	.336	2	.054	3	0	9	0	1	0
170		min	-610.348	3	.066	12	-.014	1	0	3	0	3	-.001	4
171		10	max	717.121	2	.199	2	.054	3	0	9	0	1	0
172		min	-610.4	3	-.015	3	-.014	1	0	3	0	3	-.001	4
173		11	max	717.051	2	.061	2	.054	3	0	9	0	1	0
174		min	-610.453	3	-.118	3	-.014	1	0	3	0	3	-.001	4
175		12	max	716.981	2	-.047	15	.054	3	0	9	0	1	0
176		min	-610.505	3	-.221	3	-.014	1	0	3	0	3	-.001	4
177		13	max	716.911	2	-.089	15	.054	3	0	9	0	1	0
178		min	-610.558	3	-.378	4	-.014	1	0	3	0	3	-.001	4
179		14	max	716.841	2	-.13	15	.054	3	0	9	0	1	0
180		min	-610.61	3	-.554	4	-.014	1	0	3	0	3	-.001	4
181		15	max	716.771	2	-.172	15	.054	3	0	9	0	1	0
182		min	-610.663	3	-.731	4	-.014	1	0	3	0	3	0	4
183		16	max	716.701	2	-.213	15	.054	3	0	9	0	1	0
184		min	-610.715	3	-.907	4	-.014	1	0	3	0	3	0	4
185		17	max	716.631	2	-.255	15	.054	3	0	9	0	9	0
186		min	-610.768	3	-1.083	4	-.014	1	0	3	0	3	0	4
187		18	max	716.561	2	-.296	15	.054	3	0	9	0	9	0
188		min	-610.82	3	-1.26	4	-.014	1	0	3	0	3	0	4
189		19	max	716.491	2	-.338	15	.054	3	0	9	0	9	0
190		min	-610.873	3	-1.436	4	-.014	1	0	3	0	3	0	1
191	M8	1	max	625.714	1	0	1	.126	9	0	1	0	2	0
192		min	20.181	15	0	1	-.801	3	0	1	0	3	0	1
193		2	max	625.778	1	0	1	.126	9	0	1	0	9	0
194		min	20.2	15	0	1	-.801	3	0	1	0	3	0	1
195		3	max	625.843	1	0	1	.126	9	0	1	0	9	0
196		min	20.22	15	0	1	-.801	3	0	1	0	3	0	1
197		4	max	625.908	1	0	1	.126	9	0	1	0	9	0
198		min	20.239	15	0	1	-.801	3	0	1	0	3	0	1
199		5	max	625.973	1	0	1	.126	9	0	1	0	9	0
200		min	20.259	15	0	1	-.801	3	0	1	0	3	0	1
201		6	max	626.037	1	0	1	.126	9	0	1	0	9	0
202		min	20.278	15	0	1	-.801	3	0	1	0	3	0	1
203		7	max	626.102	1	0	1	.126	9	0	1	0	9	0
204		min	20.298	15	0	1	-.801	3	0	1	0	3	0	1
205		8	max	626.167	1	0	1	.126	9	0	1	0	9	0
206		min	20.317	15	0	1	-.801	3	0	1	0	3	0	1
207		9	max	626.231	1	0	1	.126	9	0	1	0	9	0
208		min	20.337	15	0	1	-.801	3	0	1	0	3	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	626.296	1	0	1	.126	9	0	1	0	9	0	1
210		min	20.357	15	0	1	-.801	3	0	1	0	3	0	1
211	11	max	626.361	1	0	1	.126	9	0	1	0	9	0	1
212		min	20.376	15	0	1	-.801	3	0	1	0	3	0	1
213	12	max	626.426	1	0	1	.126	9	0	1	0	9	0	1
214		min	20.396	15	0	1	-.801	3	0	1	0	3	0	1
215	13	max	626.49	1	0	1	.126	9	0	1	0	9	0	1
216		min	20.415	15	0	1	-.801	3	0	1	0	3	0	1
217	14	max	626.555	1	0	1	.126	9	0	1	0	9	0	1
218		min	20.435	15	0	1	-.801	3	0	1	0	3	0	1
219	15	max	626.62	1	0	1	.126	9	0	1	0	9	0	1
220		min	20.454	15	0	1	-.801	3	0	1	-.001	3	0	1
221	16	max	626.684	1	0	1	.126	9	0	1	0	9	0	1
222		min	20.474	15	0	1	-.801	3	0	1	-.001	3	0	1
223	17	max	626.749	1	0	1	.126	9	0	1	0	9	0	1
224		min	20.493	15	0	1	-.801	3	0	1	-.001	3	0	1
225	18	max	626.814	1	0	1	.126	9	0	1	0	9	0	1
226		min	20.513	15	0	1	-.801	3	0	1	-.001	3	0	1
227	19	max	626.878	1	0	1	.126	9	0	1	0	9	0	1
228		min	20.532	15	0	1	-.801	3	0	1	-.001	3	0	1
229	M10	1	max	191.288	2	.679	.004	3	0	1	0	1	0	1
230		min	-267.797	3	.16	15	-.098	1	0	3	0	3	0	1
231	2	max	191.423	2	.621	4	.004	3	0	1	0	1	0	15
232		min	-267.696	3	.146	15	-.098	1	0	3	0	3	0	4
233	3	max	191.558	2	.564	4	.004	3	0	1	0	1	0	15
234		min	-267.595	3	.133	15	-.098	1	0	3	0	3	0	4
235	4	max	191.693	2	.506	4	.004	3	0	1	0	1	0	15
236		min	-267.494	3	.119	15	-.098	1	0	3	0	3	0	4
237	5	max	191.828	2	.449	4	.004	3	0	1	0	1	0	15
238		min	-267.393	3	.106	15	-.098	1	0	3	0	3	0	4
239	6	max	191.963	2	.391	4	.004	3	0	1	0	1	0	15
240		min	-267.292	3	.092	15	-.098	1	0	3	0	3	0	4
241	7	max	192.097	2	.334	4	.004	3	0	1	0	1	0	15
242		min	-267.19	3	.079	15	-.098	1	0	3	0	3	0	4
243	8	max	192.232	2	.276	4	.004	3	0	1	0	9	0	15
244		min	-267.089	3	.065	15	-.098	1	0	3	0	3	0	4
245	9	max	192.367	2	.219	4	.004	3	0	1	0	10	0	15
246		min	-266.988	3	.051	15	-.098	1	0	3	0	3	0	4
247	10	max	192.502	2	.161	4	.004	3	0	1	0	10	0	15
248		min	-266.887	3	.038	15	-.098	1	0	3	0	3	0	4
249	11	max	192.637	2	.111	2	.004	3	0	1	0	10	0	15
250		min	-266.786	3	.022	12	-.098	1	0	3	0	3	0	4
251	12	max	192.772	2	.066	2	.004	3	0	1	0	10	0	15
252		min	-266.685	3	-.004	3	-.098	1	0	3	0	3	0	4
253	13	max	192.907	2	.021	2	.004	3	0	1	0	10	0	15
254		min	-266.583	3	-.038	3	-.098	1	0	3	0	3	0	4
255	14	max	193.042	2	-.016	15	.004	3	0	1	0	10	0	15
256		min	-266.482	3	-.071	3	-.098	1	0	3	0	3	0	4
257	15	max	193.176	2	-.03	15	.004	3	0	1	0	10	0	15
258		min	-266.381	3	-.126	4	-.098	1	0	3	0	3	0	4
259	16	max	193.311	2	-.043	15	.004	3	0	1	0	10	0	15
260		min	-266.28	3	-.184	4	-.098	1	0	3	0	3	0	4
261	17	max	193.446	2	-.057	15	.004	3	0	1	0	10	0	15
262		min	-266.179	3	-.241	4	-.098	1	0	3	0	3	0	4
263	18	max	193.581	2	-.07	15	.004	3	0	1	0	10	0	15
264		min	-266.078	3	-.299	4	-.098	1	0	3	0	3	0	4
265	19	max	193.716	2	-.084	15	.004	3	0	1	0	10	0	15



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-265.977	3	-.356	4	-.098	1	0	3	0	3	0	4
267		1	max	232.307	2	1.736	4	.116	1	0	3	0	3	0	4
268			min	-220.267	3	.408	15	-.07	3	0	10	0	1	0	15
269		2	max	232.237	2	1.56	4	.116	1	0	3	0	3	0	2
270			min	-220.32	3	.367	15	-.07	3	0	10	0	1	0	3
271		3	max	232.167	2	1.383	4	.116	1	0	3	0	3	0	2
272			min	-220.372	3	.325	15	-.07	3	0	10	0	1	0	3
273		4	max	232.097	2	1.207	4	.116	1	0	3	0	3	0	15
274			min	-220.425	3	.284	15	-.07	3	0	10	0	1	0	4
275		5	max	232.027	2	1.03	4	.116	1	0	3	0	3	0	15
276			min	-220.477	3	.242	15	-.07	3	0	10	0	1	0	4
277		6	max	231.957	2	.854	4	.116	1	0	3	0	3	0	15
278			min	-220.53	3	.201	15	-.07	3	0	10	0	1	0	4
279		7	max	231.887	2	.678	4	.116	1	0	3	0	3	0	15
280			min	-220.582	3	.159	15	-.07	3	0	10	0	1	0	4
281		8	max	231.817	2	.501	4	.116	1	0	3	0	3	0	15
282			min	-220.635	3	.118	15	-.07	3	0	10	0	1	-.001	4
283	9	max	231.747	2	.325	4	.116	1	0	3	0	3	0	15	
284		min	-220.687	3	.076	15	-.07	3	0	10	0	1	-.001	4	
285	10	max	231.677	2	.149	4	.116	1	0	3	0	3	0	15	
286		min	-220.74	3	.035	15	-.07	3	0	10	0	1	-.001	4	
287	11	max	231.607	2	.006	2	.116	1	0	3	0	3	0	15	
288		min	-220.792	3	-.054	3	-.07	3	0	10	0	1	-.001	4	
289	12	max	231.537	2	-.048	15	.116	1	0	3	0	3	0	15	
290		min	-220.845	3	-.204	4	-.07	3	0	10	0	1	-.001	4	
291	13	max	231.467	2	-.089	15	.116	1	0	3	0	3	0	15	
292		min	-220.897	3	-.381	4	-.07	3	0	10	0	1	-.001	4	
293	14	max	231.397	2	-.131	15	.116	1	0	3	0	3	0	15	
294		min	-220.95	3	-.557	4	-.07	3	0	10	0	1	-.001	4	
295	15	max	231.327	2	-.172	15	.116	1	0	3	0	3	0	15	
296		min	-221.002	3	-.733	4	-.07	3	0	10	0	1	0	4	
297	16	max	231.257	2	-.214	15	.116	1	0	3	0	3	0	15	
298		min	-221.055	3	-.91	4	-.07	3	0	10	0	1	0	4	
299	17	max	231.187	2	-.255	15	.116	1	0	3	0	3	0	15	
300		min	-221.107	3	-1.086	4	-.07	3	0	10	0	10	0	4	
301	18	max	231.117	2	-.297	15	.116	1	0	3	0	3	0	15	
302		min	-221.159	3	-1.262	4	-.07	3	0	10	0	10	0	4	
303	19	max	231.047	2	-.338	15	.116	1	0	3	0	3	0	1	
304		min	-221.212	3	-1.439	4	-.07	3	0	10	0	10	0	1	
305	M12	1	max	229.887	1	0	1	.6	1	0	1	0	2	0	1
306		min	9.14	15	0	1	-.035	10	0	1	0	3	0	1	
307	2	max	229.952	1	0	1	.6	1	0	1	0	1	0	1	
308		min	9.159	15	0	1	-.035	10	0	1	0	15	0	1	
309	3	max	230.016	1	0	1	.6	1	0	1	0	1	0	1	
310		min	9.179	15	0	1	-.035	10	0	1	0	10	0	1	
311	4	max	230.081	1	0	1	.6	1	0	1	0	1	0	1	
312		min	9.198	15	0	1	-.035	10	0	1	0	10	0	1	
313	5	max	230.146	1	0	1	.6	1	0	1	0	1	0	1	
314		min	9.218	15	0	1	-.035	10	0	1	0	10	0	1	
315	6	max	230.21	1	0	1	.6	1	0	1	0	1	0	1	
316		min	9.237	15	0	1	-.035	10	0	1	0	10	0	1	
317	7	max	230.275	1	0	1	.6	1	0	1	0	1	0	1	
318		min	9.257	15	0	1	-.035	10	0	1	0	10	0	1	
319	8	max	230.34	1	0	1	.6	1	0	1	0	1	0	1	
320		min	9.276	15	0	1	-.035	10	0	1	0	10	0	1	
321	9	max	230.404	1	0	1	.6	1	0	1	0	1	0	1	
322		min	9.296	15	0	1	-.035	10	0	1	0	10	0	1	



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323		10	max	230.469	1	0	1	.6	1	0	1	0	1	0	1
324			min	9.315	15	0	1	-.035	10	0	1	0	10	0	1
325		11	max	230.534	1	0	1	.6	1	0	1	0	1	0	1
326			min	9.335	15	0	1	-.035	10	0	1	0	10	0	1
327		12	max	230.599	1	0	1	.6	1	0	1	0	1	0	1
328			min	9.354	15	0	1	-.035	10	0	1	0	10	0	1
329		13	max	230.663	1	0	1	.6	1	0	1	0	1	0	1
330			min	9.374	15	0	1	-.035	10	0	1	0	10	0	1
331		14	max	230.728	1	0	1	.6	1	0	1	0	1	0	1
332			min	9.393	15	0	1	-.035	10	0	1	0	10	0	1
333		15	max	230.793	1	0	1	.6	1	0	1	0	1	0	1
334			min	9.413	15	0	1	-.035	10	0	1	0	10	0	1
335		16	max	230.857	1	0	1	.6	1	0	1	0	1	0	1
336			min	9.432	15	0	1	-.035	10	0	1	0	10	0	1
337		17	max	230.922	1	0	1	.6	1	0	1	0	1	0	1
338			min	9.452	15	0	1	-.035	10	0	1	0	10	0	1
339		18	max	230.987	1	0	1	.6	1	0	1	0	1	0	1
340			min	9.471	15	0	1	-.035	10	0	1	0	10	0	1
341		19	max	231.052	1	0	1	.6	1	0	1	0	1	0	1
342			min	9.491	15	0	1	-.035	10	0	1	0	10	0	1
343	M1	1	max	72.04	1	338.104	3	.763	10	0	2	.03	1	0	2
344			min	3.387	15	-211.853	2	-15.401	1	0	3	-.002	10	0	3
345		2	max	72.2	1	337.932	3	.763	10	0	2	.027	1	.046	2
346			min	3.435	15	-212.081	2	-15.401	1	0	3	-.001	10	-.074	3
347		3	max	117.376	3	3.933	9	.761	10	0	10	.023	1	.092	2
348			min	-28.235	2	-29.955	2	-15.363	1	0	1	-.001	10	-.146	3
349		4	max	117.496	3	3.742	9	.761	10	0	10	.02	1	.098	2
350			min	-28.075	2	-30.184	2	-15.363	1	0	1	-.001	10	-.144	3
351		5	max	117.616	3	3.551	9	.761	10	0	10	.017	1	.105	2
352			min	-27.915	2	-30.412	2	-15.363	1	0	1	0	10	-.142	3
353		6	max	117.736	3	3.361	9	.761	10	0	10	.013	1	.111	2
354			min	-27.755	2	-30.641	2	-15.363	1	0	1	0	10	-.14	3
355		7	max	117.857	3	3.17	9	.761	10	0	10	.01	1	.118	2
356			min	-27.594	2	-30.87	2	-15.363	1	0	1	0	10	-.138	3
357		8	max	117.977	3	2.98	9	.761	10	0	10	.007	1	.125	2
358			min	-27.434	2	-31.099	2	-15.363	1	0	1	0	10	-.137	3
359		9	max	118.097	3	2.789	9	.761	10	0	10	.003	3	.131	2
360			min	-27.274	2	-31.327	2	-15.363	1	0	1	0	10	-.135	3
361		10	max	118.217	3	2.598	9	.761	10	0	10	.002	3	.138	2
362			min	-27.114	2	-31.556	2	-15.363	1	0	1	0	10	-.133	3
363		11	max	118.337	3	2.408	9	.761	10	0	10	0	3	.145	2
364			min	-26.954	2	-31.785	2	-15.363	1	0	1	-.003	1	-.131	3
365		12	max	118.457	3	2.217	9	.761	10	0	10	0	10	.152	2
366			min	-26.794	2	-32.014	2	-15.363	1	0	1	-.007	1	-.129	3
367		13	max	118.577	3	2.026	9	.761	10	0	10	0	10	.159	2
368			min	-26.634	2	-32.242	2	-15.363	1	0	1	-.01	1	-.127	3
369		14	max	118.697	3	1.836	9	.761	10	0	10	0	10	.166	2
370			min	-26.473	2	-32.471	2	-15.363	1	0	1	-.013	1	-.125	3
371		15	max	118.818	3	1.645	9	.761	10	0	10	0	10	.173	2
372			min	-26.313	2	-32.7	2	-15.363	1	0	1	-.017	1	-.122	3
373		16	max	83.475	2	171.24	2	.766	10	0	1	0	10	.178	2
374			min	1.645	15	-205.015	3	-15.462	1	0	3	-.02	1	-.119	3
375		17	max	83.636	2	171.011	2	.766	10	0	1	.001	10	.141	2
376			min	1.694	15	-205.187	3	-15.462	1	0	3	-.023	1	-.074	3
377		18	max	-3.434	15	326.309	2	.801	10	0	3	.001	10	.071	2
378			min	-72.212	1	-169.668	3	-16.021	1	0	2	-.027	1	-.037	3
379		19	max	-3.386	15	326.08	2	.801	10	0	3	.001	10	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-72.051	1	-169.84	3	-16.021	1	0	2	-.03	1	0	3
381	M5	1	max	181.711	1	1066.703	3	0	11	0	.013	3	0	3
382		min	-4.889	3	-657.228	2	-88.845	3	0	3	0	11	0	2
383		2	max	181.871	1	1066.531	3	0	11	0	0	9	.142	2
384		min	-4.769	3	-657.456	2	-88.845	3	0	3	-.006	3	-.231	3
385		3	max	324.361	3	4.365	9	9.57	3	0	0	9	.282	2
386		min	-81.554	2	-97.106	2	-.146	9	0	9	-.024	3	-.457	3
387		4	max	324.481	3	4.175	9	9.57	3	0	0	9	.303	2
388		min	-81.393	2	-97.334	2	-.146	9	0	9	-.022	3	-.449	3
389		5	max	324.601	3	3.984	9	9.57	3	0	0	9	.325	2
390		min	-81.233	2	-97.563	2	-.146	9	0	9	-.02	3	-.442	3
391		6	max	324.721	3	3.794	9	9.57	3	0	0	9	.346	2
392		min	-81.073	2	-97.792	2	-.146	9	0	9	-.018	3	-.434	3
393		7	max	324.841	3	3.603	9	9.57	3	0	0	9	.367	2
394		min	-80.913	2	-98.021	2	-.146	9	0	9	-.016	3	-.426	3
395		8	max	324.961	3	3.412	9	9.57	3	0	0	9	.388	2
396		min	-80.753	2	-98.249	2	-.146	9	0	9	-.014	3	-.418	3
397		9	max	325.082	3	3.222	9	9.57	3	0	0	9	.41	2
398		min	-80.593	2	-98.478	2	-.146	9	0	9	-.012	3	-.41	3
399		10	max	325.202	3	3.031	9	9.57	3	0	0	1	.431	2
400		min	-80.432	2	-98.707	2	-.146	9	0	9	-.01	3	-.402	3
401		11	max	325.322	3	2.84	9	9.57	3	0	0	2	.452	2
402		min	-80.272	2	-98.935	2	-.146	9	0	9	-.008	3	-.394	3
403		12	max	325.442	3	2.65	9	9.57	3	0	0	2	.474	2
404		min	-80.112	2	-99.164	2	-.146	9	0	9	-.006	3	-.386	3
405		13	max	325.562	3	2.459	9	9.57	3	0	0	2	.495	2
406		min	-79.952	2	-99.393	2	-.146	9	0	9	-.004	3	-.378	3
407		14	max	325.682	3	2.269	9	9.57	3	0	0	2	.517	2
408		min	-79.792	2	-99.622	2	-.146	9	0	9	-.002	3	-.37	3
409		15	max	325.802	3	2.078	9	9.57	3	0	0	3	.539	2
410		min	-79.632	2	-99.85	2	-.146	9	0	9	0	9	-.362	3
411		16	max	257.515	2	535.048	2	9.555	3	0	.002	3	.555	2
412		min	3.488	15	-577.97	3	-.147	9	0	2	0	9	-.349	3
413		17	max	257.675	2	534.82	2	9.555	3	0	.004	3	.439	2
414		min	3.536	15	-578.142	3	-.147	9	0	2	0	9	-.224	3
415		18	max	-2.745	12	1017.752	2	8.749	3	0	.006	3	.22	2
416		min	-181.856	1	-516.164	3	-.026	9	0	9	0	9	-.111	3
417		19	max	-2.665	12	1017.523	2	8.749	3	0	.008	3	0	3
418		min	-181.696	1	-516.335	3	-.026	9	0	9	0	9	0	2
419	M9	1	max	72.029	1	337.99	3	94.595	3	0	.002	10	0	2
420		min	3.379	15	-211.853	2	-.763	10	0	2	-.03	1	0	3
421		2	max	72.189	1	337.818	3	94.595	3	0	.001	10	.046	2
422		min	3.427	15	-212.081	2	-.763	10	0	2	-.027	1	-.074	3
423		3	max	116.702	3	3.928	9	15.32	1	0	.017	3	.092	2
424		min	-27.786	2	-29.926	2	-2.804	3	0	10	-.023	1	-.145	3
425		4	max	116.822	3	3.737	9	15.32	1	0	.016	3	.098	2
426		min	-27.626	2	-30.155	2	-2.804	3	0	10	-.02	1	-.144	3
427		5	max	116.942	3	3.547	9	15.32	1	0	.016	3	.105	2
428		min	-27.466	2	-30.384	2	-2.804	3	0	10	-.017	1	-.142	3
429		6	max	117.063	3	3.356	9	15.32	1	0	.015	3	.111	2
430		min	-27.306	2	-30.613	2	-2.804	3	0	10	-.013	1	-.14	3
431		7	max	117.183	3	3.166	9	15.32	1	0	.014	3	.118	2
432		min	-27.146	2	-30.841	2	-2.804	3	0	10	-.01	1	-.138	3
433		8	max	117.303	3	2.975	9	15.32	1	0	.014	3	.125	2
434		min	-26.986	2	-31.07	2	-2.804	3	0	10	-.007	1	-.136	3
435		9	max	117.423	3	2.784	9	15.32	1	0	.013	3	.131	2
436		min	-26.825	2	-31.299	2	-2.804	3	0	10	-.003	1	-.135	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	117.543	3	2.594	9	15.32	1	0	1	.013	3	.138	2
438		min	-26.665	2	-31.528	2	-2.804	3	0	10	0	1	-.133	3
439	11	max	117.663	3	2.403	9	15.32	1	0	1	.012	3	.145	2
440		min	-26.505	2	-31.756	2	-2.804	3	0	10	0	10	-.131	3
441	12	max	117.783	3	2.212	9	15.32	1	0	1	.011	3	.152	2
442		min	-26.345	2	-31.985	2	-2.804	3	0	10	0	10	-.129	3
443	13	max	117.903	3	2.022	9	15.32	1	0	1	.011	3	.159	2
444		min	-26.185	2	-32.214	2	-2.804	3	0	10	0	10	-.127	3
445	14	max	118.024	3	1.831	9	15.32	1	0	1	.013	1	.166	2
446		min	-26.025	2	-32.443	2	-2.804	3	0	10	0	10	-.125	3
447	15	max	118.144	3	1.641	9	15.32	1	0	1	.017	1	.173	2
448		min	-25.864	2	-32.671	2	-2.804	3	0	10	0	10	-.123	3
449	16	max	83.722	2	170.889	2	15.421	1	0	10	.02	1	.178	2
450		min	1.719	15	-205.737	3	-2.877	3	0	3	0	10	-.119	3
451	17	max	83.882	2	170.66	2	15.421	1	0	10	.023	1	.141	2
452		min	1.768	15	-205.909	3	-2.877	3	0	3	-.001	10	-.074	3
453	18	max	-3.426	15	326.309	2	16.034	1	0	2	.027	1	.071	2
454		min	-72.192	1	-169.654	3	-2.394	3	0	3	-.001	10	-.037	3
455	19	max	-3.378	15	326.08	2	16.034	1	0	2	.03	1	0	2
456		min	-72.032	1	-169.825	3	-2.394	3	0	3	-.001	10	0	3
457	M13	1	max	94.586	3	211.773	2	-3.379	15	0	.03	1	0	2
458		min	-.763	10	-338.058	3	-72.025	1	0	3	-.002	10	0	3
459	2	max	94.586	3	151.777	2	-2.562	15	0	2	.018	3	.121	3
460		min	-.763	10	-241.238	3	-53.926	1	0	3	-.004	2	-.076	2
461	3	max	94.586	3	91.782	2	-1.366	10	0	2	.014	3	.201	3
462		min	-.763	10	-144.419	3	-35.827	1	0	3	-.015	1	-.126	2
463	4	max	94.586	3	31.786	2	.891	10	0	2	.01	3	.241	3
464		min	-.763	10	-47.599	3	-17.728	1	0	3	-.026	1	-.152	2
465	5	max	94.586	3	49.22	3	4.73	2	0	2	.006	3	.241	3
466		min	-.763	10	-28.209	2	-7.921	3	0	3	-.029	1	-.153	2
467	6	max	94.586	3	146.04	3	18.47	1	0	2	.003	3	.2	3
468		min	-.763	10	-88.204	2	-6.732	3	0	3	-.025	1	-.129	2
469	7	max	94.586	3	242.859	3	36.569	1	0	2	0	10	.119	3
470		min	-.763	10	-148.2	2	-5.543	3	0	3	-.014	1	-.079	2
471	8	max	94.586	3	339.679	3	54.668	1	0	2	.008	2	0	9
472		min	-.763	10	-208.195	2	-4.354	3	0	3	-.002	3	-.005	2
473	9	max	94.586	3	436.498	3	72.766	1	0	2	.032	1	.094	2
474		min	-.763	10	-268.191	2	-3.165	3	0	3	-.003	3	-.164	3
475	10	max	94.586	3	-6.425	15	90.865	1	0	2	.066	1	.218	2
476		min	-.763	10	-533.318	3	1.39	12	0	3	-.017	3	-.366	3
477	11	max	15.428	1	268.191	2	4.143	3	0	3	.032	1	.094	2
478		min	-.763	10	-436.498	3	-72.755	1	0	2	-.016	3	-.164	3
479	12	max	15.428	1	208.195	2	5.332	3	0	3	.008	2	0	9
480		min	-.763	10	-339.679	3	-54.656	1	0	2	-.014	3	-.005	2
481	13	max	15.428	1	148.2	2	6.521	3	0	3	0	10	.119	3
482		min	-.763	10	-242.859	3	-36.557	1	0	2	-.014	1	-.079	2
483	14	max	15.428	1	88.204	2	7.71	3	0	3	-.001	15	.2	3
484		min	-.763	10	-146.04	3	-18.458	1	0	2	-.026	1	-.129	2
485	15	max	15.428	1	28.209	2	8.899	3	0	3	-.001	15	.241	3
486		min	-.763	10	-49.22	3	-4.73	2	0	2	-.029	1	-.153	2
487	16	max	15.428	1	47.599	3	17.739	1	0	3	0	12	.241	3
488		min	-.763	10	-31.786	2	-.89	10	0	2	-.026	1	-.152	2
489	17	max	15.428	1	144.419	3	35.838	1	0	3	.003	3	.201	3
490		min	-.763	10	-91.782	2	1.367	10	0	2	-.015	1	-.126	2
491	18	max	15.428	1	241.238	3	53.937	1	0	3	.008	3	.121	3
492		min	-.763	10	-151.777	2	2.57	15	0	2	-.004	2	-.076	2
493	19	max	15.428	1	338.058	3	72.036	1	0	3	.03	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-763	10	-211.773	2	3.387	15	0	2	-.002	10	0	3
495	M16	1	max	2.399	3	326.185	2	-3.378	15	0	3	.03	1	0	2
496			min	-16.007	1	-169.861	3	-72.037	1	0	2	-.001	10	0	3
497		2	max	2.399	3	233.351	2	-2.561	15	0	3	.005	9	.061	3
498			min	-16.007	1	-122.3	3	-53.938	1	0	2	-.004	2	-.117	2
499		3	max	2.399	3	140.516	2	-1.39	10	0	3	0	3	.102	3
500			min	-16.007	1	-74.739	3	-35.839	1	0	2	-.015	1	-.194	2
501		4	max	2.399	3	47.681	2	.867	10	0	3	-.001	15	.123	3
502			min	-16.007	1	-27.179	3	-17.74	1	0	2	-.026	1	-.234	2
503		5	max	2.399	3	20.382	3	4.691	2	0	3	-.001	15	.125	3
504			min	-16.007	1	-45.153	2	-5.187	3	0	2	-.029	1	-.234	2
505		6	max	2.399	3	67.943	3	18.458	1	0	3	-.001	15	.106	3
506			min	-16.007	1	-137.988	2	-3.998	3	0	2	-.025	1	-.196	2
507		7	max	2.399	3	115.504	3	36.557	1	0	3	0	10	.068	3
508			min	-16.007	1	-230.822	2	-2.809	3	0	2	-.014	1	-.119	2
509		8	max	2.399	3	163.064	3	54.656	1	0	3	.008	2	.01	3
510			min	-16.007	1	-323.657	2	-1.62	3	0	2	-.009	3	-.004	2
511		9	max	2.399	3	210.625	3	72.755	1	0	3	.032	1	.151	2
512			min	-16.007	1	-416.491	2	-.431	3	0	2	-.01	3	-.068	3
513		10	max	.801	10	-6.421	15	90.854	1	0	15	.066	1	.343	2
514			min	-16.007	1	-509.326	2	-2.405	3	0	2	-.009	3	-.166	3
515		11	max	.801	10	416.491	2	-1.031	12	0	2	.032	1	.151	2
516			min	-15.994	1	-210.625	3	-72.735	1	0	3	-.002	3	-.068	3
517		12	max	.801	10	323.657	2	-.027	3	0	2	.008	2	.01	3
518			min	-15.994	1	-163.064	3	-54.636	1	0	3	-.003	3	-.004	2
519		13	max	.801	10	230.822	2	1.162	3	0	2	0	10	.068	3
520			min	-15.994	1	-115.503	3	-36.537	1	0	3	-.014	1	-.119	2
521		14	max	.801	10	137.988	2	2.351	3	0	2	-.001	12	.106	3
522			min	-15.994	1	-67.943	3	-18.439	1	0	3	-.025	1	-.196	2
523		15	max	.801	10	45.153	2	3.54	3	0	2	0	12	.125	3
524			min	-15.994	1	-20.382	3	-4.691	2	0	3	-.029	1	-.234	2
525		16	max	.801	10	27.179	3	17.759	1	0	2	.001	3	.123	3
526			min	-15.994	1	-47.681	2	-.867	10	0	3	-.026	1	-.234	2
527		17	max	.801	10	74.74	3	35.858	1	0	2	.004	3	.102	3
528			min	-15.994	1	-140.516	2	1.39	10	0	3	-.015	1	-.194	2
529		18	max	.801	10	122.3	3	53.957	1	0	2	.006	3	.061	3
530			min	-15.994	1	-233.351	2	2.569	15	0	3	-.004	2	-.117	2
531		19	max	.801	10	169.861	3	72.056	1	0	2	.03	1	0	2
532			min	-15.994	1	-326.185	2	3.386	15	0	3	-.001	10	0	3
533	M15	1	max	0	1	.792	3	.146	3	0	1	0	1	0	1
534			min	-135.17	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.704	3	.146	3	0	1	0	1	0	1
536			min	-135.246	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.616	3	.146	3	0	1	0	1	0	1
538			min	-135.321	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.528	3	.146	3	0	1	0	1	0	1
540			min	-135.397	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.44	3	.146	3	0	1	0	1	0	1
542			min	-135.473	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.352	3	.146	3	0	1	0	1	0	1
544			min	-135.548	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.264	3	.146	3	0	1	0	3	0	1
546			min	-135.624	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.176	3	.146	3	0	1	0	3	0	1
548			min	-135.699	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.088	3	.146	3	0	1	0	3	0	1
550			min	-135.775	3	0	1	0	1	0	3	0	1	-.001	3

***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	1	0	1	.146	3	0	1	0	3	0	1
552			min	-135.85	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.146	3	0	1	0	3	0	1
554			min	-135.926	3	-.088	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.146	3	0	1	0	3	0	1
556			min	-136.001	3	-.176	3	0	1	0	3	0	1	0	3
557		13	max	0	1	0	1	.146	3	0	1	0	3	0	1
558			min	-136.077	3	-.264	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.146	3	0	1	0	3	0	1
560			min	-136.152	3	-.352	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.146	3	0	1	0	3	0	1
562			min	-136.228	3	-.44	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.146	3	0	1	0	3	0	1
564			min	-136.303	3	-.528	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.146	3	0	1	0	3	0	1
566			min	-136.379	3	-.616	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.146	3	0	1	0	3	0	1
568			min	-136.454	3	-.704	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.146	3	0	1	0	3	0	1
570			min	-136.53	3	-.792	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	1.356	4	.036	1	0	3	0	3	0	1
572			min	-134.555	3	0	2	-.061	3	0	1	0	1	0	1
573		2	max	0	2	1.205	4	.036	1	0	3	0	3	0	2
574			min	-134.479	3	0	2	-.061	3	0	1	0	1	0	4
575		3	max	0	2	1.054	4	.036	1	0	3	0	3	0	2
576			min	-134.404	3	0	2	-.061	3	0	1	0	1	0	4
577		4	max	0	2	.904	4	.036	1	0	3	0	3	0	2
578			min	-134.328	3	0	2	-.061	3	0	1	0	1	0	4
579		5	max	0	2	.753	4	.036	1	0	3	0	3	0	2
580			min	-134.253	3	0	2	-.061	3	0	1	0	1	-.001	4
581		6	max	0	2	.602	4	.036	1	0	3	0	3	0	2
582			min	-134.177	3	0	2	-.061	3	0	1	0	1	-.001	4
583		7	max	0	2	.452	4	.036	1	0	3	0	3	0	2
584			min	-134.102	3	0	2	-.061	3	0	1	0	1	-.002	4
585		8	max	0	2	.301	4	.036	1	0	3	0	3	0	2
586			min	-134.026	3	0	2	-.061	3	0	1	0	1	-.002	4
587		9	max	0	2	.151	4	.036	1	0	3	0	3	0	2
588			min	-133.951	3	0	2	-.061	3	0	1	0	1	-.002	4
589		10	max	0	2	0	1	.036	1	0	3	0	3	0	2
590			min	-133.875	3	0	1	-.061	3	0	1	0	1	-.002	4
591		11	max	.04	1	0	2	.036	1	0	3	0	3	0	2
592			min	-133.8	3	-.151	4	-.061	3	0	1	0	1	-.002	4
593		12	max	.14	1	0	2	.036	1	0	3	0	3	0	2
594			min	-133.724	3	-.301	4	-.061	3	0	1	0	1	-.002	4
595		13	max	.241	1	0	2	.036	1	0	3	0	1	0	2
596			min	-133.648	3	-.452	4	-.061	3	0	1	0	3	-.002	4
597		14	max	.342	1	0	2	.036	1	0	3	0	1	0	2
598			min	-133.573	3	-.602	4	-.061	3	0	1	0	3	-.001	4
599		15	max	.442	1	0	2	.036	1	0	3	0	1	0	2
600			min	-133.497	3	-.753	4	-.061	3	0	1	0	3	-.001	4
601		16	max	.543	1	0	2	.036	1	0	3	0	1	0	2
602			min	-133.422	3	-.904	4	-.061	3	0	1	0	3	0	4
603		17	max	.671	4	0	2	.036	1	0	3	0	1	0	2
604			min	-133.346	3	-1.054	4	-.061	3	0	1	0	3	0	4
605		18	max	.801	4	0	2	.036	1	0	3	0	1	0	2
606			min	-133.271	3	-1.205	4	-.061	3	0	1	0	3	0	4
607		19	max	.93	4	0	2	.036	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-133.195	3	-1.356	4	-.061	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.011	2	.002	9	1.352e-5	10	NC	3	NC	1	
2		min	-.004	3	-.011	3	-.003	3	-2.543e-4	1	3980.894	2	NC	1		
3		2	max	.002	2	.01	2	.002	9	1.287e-5	10	NC	3	NC	1	
4		min	-.004	3	-.011	3	-.002	3	-2.422e-4	1	4355.688	2	NC	1		
5		3	max	.002	2	.009	2	.002	9	1.222e-5	10	NC	3	NC	1	
6		min	-.003	3	-.01	3	-.002	3	-2.301e-4	1	4803.479	2	NC	1		
7		4	max	.002	2	.008	2	.002	9	1.158e-5	10	NC	1	NC	1	
8		min	-.003	3	-.01	3	-.002	3	-2.181e-4	1	5342.2	2	NC	1		
9		5	max	.002	2	.007	2	.002	9	1.093e-5	10	NC	1	NC	1	
10		min	-.003	3	-.009	3	-.002	3	-2.06e-4	1	5995.845	2	NC	1		
11		6	max	.001	2	.006	2	.001	9	1.029e-5	10	NC	1	NC	1	
12		min	-.003	3	-.009	3	-.002	3	-1.939e-4	1	6797.034	2	NC	1		
13		7	max	.001	2	.005	2	.001	9	9.639e-6	10	NC	1	NC	1	
14		min	-.003	3	-.008	3	-.001	3	-1.819e-4	1	7790.966	2	NC	1		
15		8	max	.001	2	.005	2	.001	9	8.992e-6	10	NC	1	NC	1	
16		min	-.002	3	-.008	3	-.001	3	-1.698e-4	1	9041.659	2	NC	1		
17		9	max	.001	2	.004	2	0	9	8.346e-6	10	NC	1	NC	1	
18		min	-.002	3	-.007	3	0	3	-1.577e-4	1	NC	1	NC	1		
19		10	max	.001	2	.003	2	0	9	7.7e-6	10	NC	1	NC	1	
20		min	-.002	3	-.007	3	0	3	-1.456e-4	1	NC	1	NC	1		
21		11	max	0	2	.003	2	0	9	7.053e-6	10	NC	1	NC	1	
22		min	-.002	3	-.006	3	0	3	-1.336e-4	1	NC	1	NC	1		
23		12	max	0	2	.002	2	0	9	6.407e-6	10	NC	1	NC	1	
24		min	-.001	3	-.005	3	0	3	-1.215e-4	1	NC	1	NC	1		
25		13	max	0	2	.002	2	0	9	5.76e-6	10	NC	1	NC	1	
26		min	-.001	3	-.005	3	0	3	-1.094e-4	1	NC	1	NC	1		
27		14	max	0	2	.001	2	0	9	5.114e-6	10	NC	1	NC	1	
28		min	-.001	3	-.004	3	0	3	-9.735e-5	1	NC	1	NC	1		
29		15	max	0	2	0	2	0	9	4.467e-6	10	NC	1	NC	1	
30		min	0	3	-.003	3	0	3	-8.528e-5	1	NC	1	NC	1		
31		16	max	0	2	0	2	0	9	3.821e-6	10	NC	1	NC	1	
32		min	0	3	-.002	3	0	3	-7.32e-5	1	NC	1	NC	1		
33		17	max	0	2	0	2	0	9	3.174e-6	10	NC	1	NC	1	
34		min	0	3	-.002	3	0	3	-6.113e-5	1	NC	1	NC	1		
35		18	max	0	2	0	2	0	9	2.528e-6	10	NC	1	NC	1	
36		min	0	3	0	3	0	3	-4.906e-5	1	NC	1	NC	1		
37		19	max	0	1	0	1	0	1	1.881e-6	10	NC	1	NC	1	
38		min	0	1	0	1	0	1	-3.742e-5	9	NC	1	NC	1		
39		M3	1	max	0	1	0	1	0	1	1.793e-5	9	NC	1	NC	1
40			min	0	1	0	1	0	1	-9.062e-7	10	NC	1	NC	1	
41	2		max	0	3	0	2	0	10	2.462e-5	1	NC	1	NC	1	
42	min		0	2	0	3	0	9	-1.204e-6	10	NC	1	NC	1		
43	3		max	0	3	0	2	0	3	3.145e-5	1	NC	1	NC	1	
44	min		0	2	-.002	3	0	9	-1.501e-6	10	NC	1	NC	1		
45	4		max	0	3	0	2	0	3	3.828e-5	1	NC	1	NC	1	
46	min		0	2	-.003	3	0	9	-1.799e-6	10	NC	1	NC	1		
47	5		max	0	3	0	2	0	3	4.511e-5	1	NC	1	NC	1	
48	min		0	2	-.004	3	0	9	-2.096e-6	10	NC	1	NC	1		
49	6		max	0	3	0	2	0	3	5.193e-5	1	NC	1	NC	1	
50	min		0	2	-.004	3	0	9	-2.394e-6	10	NC	1	NC	1		
51	7	max	0	3	0	2	0	3	5.876e-5	1	NC	1	NC	1		



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	-2.691e-6	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	6.559e-5	1	NC	1	NC	1
54			min	-.001	2	-.006	3	0	9	-2.989e-6	10	NC	1	NC	1
55		9	max	.001	3	.001	2	0	3	7.241e-5	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	10	-3.286e-6	10	NC	1	NC	1
57		10	max	.001	3	.002	2	0	3	7.924e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-3.584e-6	10	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	8.607e-5	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-3.881e-6	10	NC	1	NC	1
61		12	max	.002	3	.003	2	0	1	9.29e-5	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	10	-4.179e-6	10	NC	1	NC	1
63		13	max	.002	3	.004	2	0	1	9.972e-5	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	10	-4.476e-6	10	NC	1	NC	1
65		14	max	.002	3	.004	2	.001	1	1.065e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	10	-4.774e-6	10	NC	1	NC	1
67		15	max	.002	3	.005	2	.001	1	1.134e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	10	-5.071e-6	10	8671.563	2	NC	1
69		16	max	.002	3	.006	2	.002	1	1.202e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	10	-5.369e-6	10	7351	2	NC	1
71		17	max	.002	3	.007	2	.002	1	1.27e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-5.666e-6	10	6329.132	2	NC	1
73		18	max	.002	3	.008	2	.002	1	1.339e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-5.964e-6	10	5529.215	2	NC	1
75		19	max	.002	3	.009	2	.002	1	1.407e-4	1	NC	3	NC	1
76			min	-.003	2	-.009	3	0	10	-6.261e-6	10	4897.347	2	NC	1
77	M4	1	max	.001	1	.012	2	0	10	8.404e-6	10	NC	1	NC	1
78			min	0	15	-.011	3	-.002	1	-1.793e-4	1	NC	1	NC	1
79		2	max	.001	1	.012	2	0	10	8.404e-6	10	NC	1	NC	1
80			min	0	15	-.01	3	-.002	1	-1.793e-4	1	NC	1	NC	1
81		3	max	0	1	.011	2	0	10	8.404e-6	10	NC	1	NC	1
82			min	0	15	-.01	3	-.001	1	-1.793e-4	1	NC	1	NC	1
83		4	max	0	1	.01	2	0	10	8.404e-6	10	NC	1	NC	1
84			min	0	15	-.009	3	-.001	1	-1.793e-4	1	NC	1	NC	1
85		5	max	0	1	.01	2	0	10	8.404e-6	10	NC	1	NC	1
86			min	0	15	-.009	3	-.001	1	-1.793e-4	1	NC	1	NC	1
87		6	max	0	1	.009	2	0	10	8.404e-6	10	NC	1	NC	1
88			min	0	15	-.008	3	-.001	1	-1.793e-4	1	NC	1	NC	1
89		7	max	0	1	.008	2	0	10	8.404e-6	10	NC	1	NC	1
90			min	0	15	-.007	3	0	1	-1.793e-4	1	NC	1	NC	1
91		8	max	0	1	.008	2	0	10	8.404e-6	10	NC	1	NC	1
92			min	0	15	-.007	3	0	1	-1.793e-4	1	NC	1	NC	1
93		9	max	0	1	.007	2	0	10	8.404e-6	10	NC	1	NC	1
94			min	0	15	-.006	3	0	1	-1.793e-4	1	NC	1	NC	1
95		10	max	0	1	.006	2	0	10	8.404e-6	10	NC	1	NC	1
96			min	0	15	-.005	3	0	1	-1.793e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	10	8.404e-6	10	NC	1	NC	1
98			min	0	15	-.005	3	0	1	-1.793e-4	1	NC	1	NC	1
99		12	max	0	1	.005	2	0	10	8.404e-6	10	NC	1	NC	1
100			min	0	15	-.004	3	0	1	-1.793e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	10	8.404e-6	10	NC	1	NC	1
102			min	0	15	-.004	3	0	1	-1.793e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	10	8.404e-6	10	NC	1	NC	1
104			min	0	15	-.003	3	0	1	-1.793e-4	1	NC	1	NC	1
105		15	max	0	1	.003	2	0	10	8.404e-6	10	NC	1	NC	1
106			min	0	15	-.002	3	0	1	-1.793e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	10	8.404e-6	10	NC	1	NC	1
108			min	0	15	-.002	3	0	1	-1.793e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	8.404e-6	10	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-1.793e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	8.404e-6	10	NC	1	NC	1
112			min	0	15	0	3	0	1	-1.793e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	8.404e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.793e-4	1	NC	1	NC	1
115	M6	1	max	.006	2	.033	2	0	9	5.524e-4	3	NC	3	NC	1
116			min	-.011	3	-.032	3	-.007	3	-3.473e-7	9	1282.548	2	5766.329	3
117		2	max	.006	2	.031	2	0	9	5.345e-4	3	NC	3	NC	1
118			min	-.01	3	-.031	3	-.007	3	-1.17e-6	1	1374.707	2	6091.115	3
119		3	max	.005	2	.029	2	0	9	5.166e-4	3	NC	3	NC	1
120			min	-.01	3	-.029	3	-.007	3	-2.117e-6	1	1480.616	2	6481.651	3
121		4	max	.005	2	.026	2	0	9	4.988e-4	3	NC	3	NC	1
122			min	-.009	3	-.027	3	-.006	3	-3.063e-6	1	1603.032	2	6951.335	3
123		5	max	.005	2	.024	2	0	9	4.809e-4	3	NC	3	NC	1
124			min	-.009	3	-.026	3	-.006	3	-4.01e-6	1	1745.504	2	7518.078	3
125		6	max	.004	2	.022	2	0	9	4.631e-4	3	NC	3	NC	1
126			min	-.008	3	-.024	3	-.005	3	-4.957e-6	1	1912.681	2	8206.112	3
127		7	max	.004	2	.02	2	0	9	4.452e-4	3	NC	3	NC	1
128			min	-.007	3	-.022	3	-.005	3	-5.904e-6	1	2110.761	2	9048.738	3
129		8	max	.004	2	.018	2	0	9	4.273e-4	3	NC	3	NC	1
130			min	-.007	3	-.02	3	-.004	3	-6.851e-6	1	2348.201	2	NC	1
131		9	max	.003	2	.016	2	0	9	4.095e-4	3	NC	3	NC	1
132			min	-.006	3	-.019	3	-.004	3	-7.798e-6	1	2636.835	2	NC	1
133		10	max	.003	2	.014	2	0	9	3.916e-4	3	NC	3	NC	1
134			min	-.006	3	-.017	3	-.003	3	-8.745e-6	1	2993.745	2	NC	1
135		11	max	.003	2	.012	2	0	9	3.738e-4	3	NC	3	NC	1
136			min	-.005	3	-.015	3	-.003	3	-9.692e-6	1	3444.541	2	NC	1
137		12	max	.002	2	.011	2	0	9	3.559e-4	3	NC	3	NC	1
138			min	-.004	3	-.013	3	-.002	3	-1.064e-5	1	4029.434	2	NC	1
139		13	max	.002	2	.009	2	0	9	3.38e-4	3	NC	3	NC	1
140			min	-.004	3	-.011	3	-.002	3	-1.159e-5	1	4815.402	2	NC	1
141		14	max	.002	2	.007	2	0	9	3.202e-4	3	NC	1	NC	1
142			min	-.003	3	-.009	3	-.001	3	-1.253e-5	1	5922.931	2	NC	1
143		15	max	.001	2	.006	2	0	9	3.023e-4	3	NC	1	NC	1
144			min	-.002	3	-.008	3	-.001	3	-1.348e-5	1	7592.879	2	NC	1
145		16	max	.001	2	.004	2	0	9	2.845e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-1.443e-5	1	NC	1	NC	1
147		17	max	0	2	.003	2	0	9	2.666e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-1.537e-5	1	NC	1	NC	1
149		18	max	0	2	.001	2	0	9	2.487e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-1.632e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.309e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.727e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	8.276e-6	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.099e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	7.936e-6	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-8.231e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	.001	3	7.596e-6	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-5.47e-5	3	NC	1	NC	1
159		4	max	.001	3	.004	2	.001	3	7.257e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.708e-5	3	NC	1	NC	1
161		5	max	.002	3	.005	2	.002	3	6.917e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	9	0	2	8951.288	2	NC	1
163		6	max	.002	3	.006	2	.002	3	2.816e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	9	0	10	7159.011	2	NC	1
165		7	max	.002	3	.008	2	.003	3	5.578e-5	3	NC	1	NC	1



***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	9	-1.275e-7	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.809e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	9	-1.275e-7	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.809e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.275e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.809e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.011	2	0	10	2.492e-4	1	NC	3	NC	1
230			min	-.003	3	-.011	3	-.002	1	-6.319e-4	3	3984.194	2	NC	1
231		2	max	.002	2	.01	2	0	10	2.372e-4	1	NC	3	NC	1
232			min	-.003	3	-.01	3	-.002	1	-6.091e-4	3	4359.417	2	NC	1
233		3	max	.002	2	.009	2	0	10	2.252e-4	1	NC	3	NC	1
234			min	-.002	3	-.01	3	-.002	1	-5.862e-4	3	4807.745	2	NC	1
235		4	max	.002	2	.008	2	0	3	2.132e-4	1	NC	1	NC	1
236			min	-.002	3	-.01	3	-.001	1	-5.634e-4	3	5347.144	2	NC	1
237		5	max	.002	2	.007	2	0	3	2.013e-4	1	NC	1	NC	1
238			min	-.002	3	-.009	3	-.001	1	-5.405e-4	3	6001.653	2	NC	1
239		6	max	.001	2	.006	2	0	3	1.893e-4	1	NC	1	NC	1
240			min	-.002	3	-.009	3	-.001	1	-5.177e-4	3	6803.96	2	NC	1
241		7	max	.001	2	.005	2	0	3	1.773e-4	1	NC	1	NC	1
242			min	-.002	3	-.008	3	-.001	1	-4.948e-4	3	7799.359	2	NC	1
243		8	max	.001	2	.005	2	0	3	1.654e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	0	1	-4.72e-4	3	9052.013	2	NC	1
245		9	max	.001	2	.004	2	0	3	1.534e-4	1	NC	1	NC	1
246			min	-.002	3	-.007	3	0	1	-4.491e-4	3	NC	1	NC	1
247		10	max	.001	2	.003	2	0	3	1.414e-4	1	NC	1	NC	1
248			min	-.001	3	-.007	3	0	1	-4.263e-4	3	NC	1	NC	1
249		11	max	0	2	.003	2	0	3	1.295e-4	1	NC	1	NC	1
250			min	-.001	3	-.006	3	0	1	-4.034e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	1.175e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-3.806e-4	3	NC	1	NC	1
253		13	max	0	2	.002	2	0	3	1.055e-4	1	NC	1	NC	1
254			min	0	3	-.005	3	0	1	-3.577e-4	3	NC	1	NC	1
255		14	max	0	2	.001	2	0	3	9.354e-5	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-3.349e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	8.157e-5	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-3.12e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	6.96e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.892e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	5.762e-5	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-2.663e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	4.565e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.435e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.368e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.206e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.057e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.621e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.928e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.336e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.29e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-3.051e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	1	2.653e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-3.766e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	1	2.139e-6	10	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-4.481e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	2.447e-6	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-5.195e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	2.756e-6	10	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-5.91e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	3.064e-6	10	NC	1	NC	1
282			min	-.001	2	-.006	3	-.003	3	-7.898e-5	3	NC	1	NC	1
283		9	max	.001	3	.001	2	0	10	3.372e-6	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	3	-1.054e-4	3	NC	1	NC	1
285		10	max	.001	3	.002	2	0	10	3.681e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.317e-4	3	NC	1	NC	1
287		11	max	.001	3	.002	2	0	10	3.989e-6	10	NC	1	NC	1
288			min	-.001	2	-.008	3	-.003	3	-1.581e-4	3	NC	1	NC	1
289		12	max	.002	3	.003	2	0	10	4.298e-6	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	3	-1.845e-4	3	NC	1	NC	1
291		13	max	.002	3	.004	2	0	10	4.606e-6	10	NC	1	NC	1
292			min	-.002	2	-.008	3	-.003	3	-2.109e-4	3	NC	1	NC	1
293		14	max	.002	3	.004	2	0	10	4.915e-6	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.003	3	-2.372e-4	3	NC	1	NC	1
295		15	max	.002	3	.005	2	0	10	5.223e-6	10	NC	1	NC	1
296			min	-.002	2	-.009	3	-.003	3	-2.636e-4	3	8683.311	2	NC	1
297		16	max	.002	3	.006	2	0	10	5.531e-6	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	3	-2.9e-4	3	7360.037	2	NC	1
299		17	max	.002	3	.007	2	0	10	5.84e-6	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.003	3	-3.164e-4	3	6336.272	2	NC	1
301		18	max	.002	3	.008	2	0	10	6.148e-6	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.003	3	-3.427e-4	3	5535.001	2	NC	1
303		19	max	.003	3	.009	2	0	10	6.457e-6	10	NC	3	NC	1
304			min	-.003	2	-.009	3	-.003	3	-3.691e-4	3	4902.149	2	NC	1
305	M12	1	max	.001	1	.012	2	.002	1	4.249e-4	3	NC	1	NC	1
306			min	0	15	-.011	3	0	10	-8.66e-6	10	NC	1	NC	1
307		2	max	.001	1	.012	2	.002	1	4.249e-4	3	NC	1	NC	1
308			min	0	15	-.01	3	0	10	-8.66e-6	10	NC	1	NC	1
309		3	max	0	1	.011	2	.002	1	4.249e-4	3	NC	1	NC	1
310			min	0	15	-.01	3	0	10	-8.66e-6	10	NC	1	NC	1
311		4	max	0	1	.01	2	.001	1	4.249e-4	3	NC	1	NC	1
312			min	0	15	-.009	3	0	10	-8.66e-6	10	NC	1	NC	1
313		5	max	0	1	.01	2	.001	1	4.249e-4	3	NC	1	NC	1
314			min	0	15	-.009	3	0	10	-8.66e-6	10	NC	1	NC	1
315		6	max	0	1	.009	2	.001	1	4.249e-4	3	NC	1	NC	1
316			min	0	15	-.008	3	0	10	-8.66e-6	10	NC	1	NC	1
317		7	max	0	1	.008	2	.001	1	4.249e-4	3	NC	1	NC	1
318			min	0	15	-.007	3	0	10	-8.66e-6	10	NC	1	NC	1
319		8	max	0	1	.008	2	0	1	4.249e-4	3	NC	1	NC	1
320			min	0	15	-.007	3	0	10	-8.66e-6	10	NC	1	NC	1
321		9	max	0	1	.007	2	0	1	4.249e-4	3	NC	1	NC	1
322			min	0	15	-.006	3	0	10	-8.66e-6	10	NC	1	NC	1
323		10	max	0	1	.006	2	0	1	4.249e-4	3	NC	1	NC	1
324			min	0	15	-.006	3	0	10	-8.66e-6	10	NC	1	NC	1
325		11	max	0	1	.005	2	0	1	4.249e-4	3	NC	1	NC	1
326			min	0	15	-.005	3	0	10	-8.66e-6	10	NC	1	NC	1
327		12	max	0	1	.005	2	0	1	4.249e-4	3	NC	1	NC	1
328			min	0	15	-.004	3	0	10	-8.66e-6	10	NC	1	NC	1
329		13	max	0	1	.004	2	0	1	4.249e-4	3	NC	1	NC	1
330			min	0	15	-.004	3	0	10	-8.66e-6	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	4.249e-4	3	NC	1	NC	1
332			min	0	15	-.003	3	0	10	-8.66e-6	10	NC	1	NC	1
333		15	max	0	1	.003	2	0	1	4.249e-4	3	NC	1	NC	1
334			min	0	15	-.002	3	0	10	-8.66e-6	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	4.249e-4	3	NC	1	NC	1
336			min	0	15	-.002	3	0	10	-8.66e-6	10	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	4.249e-4	3	NC	1	NC	1
338			min	0	15	-.001	3	0	10	-8.66e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.249e-4	3	NC	1	NC	1
340			min	0	15	0	3	0	10	-8.66e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.249e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-8.66e-6	10	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.004	3	5.275e-3	2	NC	1	NC	1
344			min	-.01	2	-.022	2	0	9	-7.886e-3	3	NC	1	NC	1
345		2	max	.01	3	.016	3	.003	3	2.604e-3	2	NC	4	NC	1
346			min	-.01	2	-.013	2	-.002	9	-3.888e-3	3	5438.194	2	NC	1
347		3	max	.01	3	.007	3	.003	3	3.67e-5	3	NC	4	NC	1
348			min	-.01	2	-.005	2	-.002	9	-1.219e-4	1	2788.467	2	NC	1
349		4	max	.01	3	.002	2	.002	3	3.94e-5	3	NC	4	NC	1
350			min	-.01	2	-.002	3	-.002	9	-1.022e-4	1	1903.094	3	NC	1
351		5	max	.009	3	.009	2	.002	3	4.211e-5	3	NC	4	NC	1
352			min	-.01	2	-.008	3	-.003	9	-8.255e-5	1	1487.615	3	NC	1
353		6	max	.009	3	.014	2	.001	3	4.481e-5	3	NC	4	NC	1
354			min	-.01	2	-.014	3	-.002	9	-6.624e-5	9	1264.096	3	NC	1
355		7	max	.009	3	.019	2	.001	3	4.752e-5	3	NC	4	NC	1
356			min	-.01	2	-.018	3	-.002	9	-5.041e-5	9	1133.226	3	NC	1
357		8	max	.009	3	.022	2	.001	3	5.022e-5	3	NC	4	NC	1
358			min	-.01	2	-.021	3	-.002	9	-3.459e-5	9	1056.358	3	NC	1
359		9	max	.009	3	.024	2	.001	3	5.292e-5	3	NC	4	NC	1
360			min	-.01	2	-.023	3	-.001	9	-1.877e-5	9	1016.174	3	NC	1
361		10	max	.009	3	.025	2	.001	3	5.563e-5	3	NC	4	NC	1
362			min	-.01	2	-.024	3	0	9	-2.941e-6	9	1004.908	3	NC	1
363		11	max	.009	3	.025	2	.001	3	5.833e-5	3	NC	4	NC	1
364			min	-.01	2	-.023	3	0	9	-1.193e-6	10	1012.19	2	NC	1
365		12	max	.009	3	.023	2	.001	3	6.104e-5	3	NC	4	NC	1
366			min	-.01	2	-.021	3	0	10	-2.224e-6	10	1049.831	2	NC	1
367		13	max	.009	3	.02	2	.002	1	7.504e-5	1	NC	4	NC	1
368			min	-.01	2	-.018	3	0	10	-3.255e-6	10	1127.022	2	NC	1
369		14	max	.009	3	.015	2	.002	1	9.474e-5	1	NC	4	NC	1
370			min	-.01	2	-.014	3	0	10	-4.286e-6	10	1264.43	2	NC	1
371		15	max	.009	3	.009	2	.002	1	1.144e-4	1	NC	4	NC	1
372			min	-.01	2	-.008	3	0	10	-5.317e-6	10	1497.294	3	NC	1
373		16	max	.009	3	.002	2	.002	1	1.286e-4	1	NC	4	NC	1
374			min	-.01	2	-.002	3	0	10	-6.072e-6	10	1891.473	3	NC	1
375		17	max	.009	3	.006	3	.002	1	8.819e-5	3	NC	4	NC	1
376			min	-.01	2	-.008	2	0	10	-1.995e-5	9	2744.708	3	NC	1
377		18	max	.009	3	.014	3	.001	3	3.921e-3	2	NC	1	NC	1
378			min	-.01	2	-.018	2	0	10	-2.191e-3	3	5382.11	3	NC	1
379		19	max	.009	3	.023	3	0	3	7.913e-3	2	NC	1	NC	1
380			min	-.01	2	-.03	2	0	9	-4.513e-3	3	5767.669	2	NC	1
381	M5	1	max	.028	3	.081	3	.004	3	1.278e-5	3	NC	1	NC	1
382			min	-.03	2	-.067	2	0	9	0	1	3923.823	3	NC	1
383		2	max	.028	3	.049	3	.006	3	1.633e-4	3	NC	4	NC	1
384			min	-.03	2	-.04	2	0	9	-1.157e-5	9	1751.394	2	NC	1
385		3	max	.028	3	.019	3	.007	3	3.108e-4	3	NC	5	NC	1
386			min	-.03	2	-.015	2	0	9	-2.305e-5	9	897.773	2	NC	1
387		4	max	.028	3	.008	2	.008	3	2.981e-4	3	NC	5	NC	1
388			min	-.03	2	-.006	3	0	9	-2.199e-5	9	626.075	3	NC	1
389		5	max	.028	3	.028	2	.009	3	2.854e-4	3	NC	5	NC	1
390			min	-.03	2	-.027	3	0	9	-2.094e-5	9	489.08	3	8415.383	3
391		6	max	.028	3	.044	2	.01	3	2.727e-4	3	NC	5	NC	1
392			min	-.03	2	-.044	3	0	9	-1.988e-5	9	415.739	3	7602.045	3
393		7	max	.027	3	.058	2	.01	3	2.6e-4	3	NC	5	NC	1







Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.009	3	.006	3	0	10	1.461e-4	3	NC	4	NC	1
452		min	-.01	2	-.008	2	-.002	1	-4.601e-5	9	2720.169	3	NC	1
453	18	max	.009	3	.014	3	0	10	2.309e-3	3	NC	1	NC	1
454		min	-.01	2	-.018	2	-.001	9	-3.921e-3	2	5334.938	3	NC	1
455	19	max	.009	3	.023	3	0	3	4.508e-3	3	NC	1	NC	1
456		min	-.01	2	-.03	2	0	9	-7.913e-3	2	5782.938	2	NC	1
457	M13	1	max	0	.026	3	.01	3	3.897e-3	3	NC	1	NC	1
458		min	-.004	3	-.022	2	-.01	2	-3.274e-3	2	NC	1	NC	1
459	2	max	0	9	.064	3	.008	3	4.806e-3	3	NC	4	NC	1
460		min	-.004	3	-.047	2	-.009	2	-4.04e-3	2	2351.071	3	NC	1
461	3	max	0	9	.096	3	.008	3	5.715e-3	3	NC	4	NC	1
462		min	-.004	3	-.069	2	-.009	2	-4.806e-3	2	1271.335	3	NC	1
463	4	max	0	9	.119	3	.009	9	6.625e-3	3	NC	4	NC	2
464		min	-.004	3	-.085	2	-.01	2	-5.572e-3	2	961.715	3	8259.21	1
465	5	max	0	9	.13	3	.012	3	7.534e-3	3	NC	4	NC	2
466		min	-.004	3	-.094	2	-.012	2	-6.337e-3	2	859.711	3	8078.147	1
467	6	max	0	9	.13	3	.015	3	8.443e-3	3	NC	4	NC	1
468		min	-.004	3	-.095	2	-.016	2	-7.103e-3	2	864.396	3	9505.493	9
469	7	max	0	9	.12	3	.018	3	9.352e-3	3	NC	4	NC	1
470		min	-.004	3	-.09	2	-.02	2	-7.869e-3	2	958.659	3	8664.536	2
471	8	max	0	9	.104	3	.022	3	1.026e-2	3	NC	4	NC	1
472		min	-.004	3	-.081	2	-.025	2	-8.635e-3	2	1154.48	3	6025.204	2
473	9	max	0	9	.088	3	.025	3	1.117e-2	3	NC	4	NC	4
474		min	-.004	3	-.071	2	-.028	2	-9.4e-3	2	1444.265	3	4818.149	2
475	10	max	0	9	.081	3	.028	3	1.208e-2	3	NC	4	NC	4
476		min	-.004	3	-.067	2	-.03	2	-1.017e-2	2	1639.106	3	4436.965	2
477	11	max	0	9	.088	3	.03	3	1.117e-2	3	NC	4	NC	4
478		min	-.004	3	-.071	2	-.028	2	-9.4e-3	2	1444.263	3	4479.996	3
479	12	max	0	9	.104	3	.03	3	1.027e-2	3	NC	4	NC	1
480		min	-.004	3	-.081	2	-.025	2	-8.635e-3	2	1154.478	3	4423.206	3
481	13	max	0	9	.12	3	.029	3	9.362e-3	3	NC	4	NC	1
482		min	-.004	3	-.09	2	-.02	2	-7.869e-3	2	958.658	3	4692.611	3
483	14	max	0	9	.13	3	.026	3	8.457e-3	3	NC	4	NC	1
484		min	-.004	3	-.095	2	-.016	2	-7.103e-3	2	864.395	3	5344.838	3
485	15	max	0	9	.131	3	.023	3	7.551e-3	3	NC	4	NC	2
486		min	-.004	3	-.094	2	-.012	2	-6.338e-3	2	859.71	3	6602.73	3
487	16	max	0	9	.12	3	.019	3	6.645e-3	3	NC	4	NC	2
488		min	-.004	3	-.085	2	-.01	2	-5.572e-3	2	961.714	3	8260.05	1
489	17	max	0	9	.097	3	.016	3	5.739e-3	3	NC	4	NC	1
490		min	-.004	3	-.069	2	-.009	2	-4.806e-3	2	1271.333	3	NC	1
491	18	max	0	9	.065	3	.012	3	4.833e-3	3	NC	4	NC	1
492		min	-.004	3	-.047	2	-.009	2	-4.04e-3	2	2351.068	3	NC	1
493	19	max	0	9	.027	3	.01	3	3.928e-3	3	NC	1	NC	1
494		min	-.004	3	-.022	2	-.01	2	-3.275e-3	2	NC	1	NC	1
495	M16	1	max	0	.023	3	.009	3	4.321e-3	2	NC	1	NC	1
496		min	0	3	-.03	2	-.01	2	-3.346e-3	3	NC	1	NC	1
497	2	max	0	9	.045	3	.012	3	5.335e-3	2	NC	4	NC	1
498		min	0	3	-.068	2	-.009	2	-4.082e-3	3	2344.07	2	NC	1
499	3	max	0	9	.064	3	.015	3	6.349e-3	2	NC	4	NC	1
500		min	0	3	-.101	2	-.009	2	-4.819e-3	3	1263.76	2	NC	1
501	4	max	0	9	.078	3	.019	3	7.363e-3	2	NC	4	NC	2
502		min	0	3	-.124	2	-.01	2	-5.555e-3	3	951.038	2	8292.942	1
503	5	max	0	9	.086	3	.022	3	8.378e-3	2	NC	4	NC	2
504		min	0	3	-.136	2	-.012	2	-6.291e-3	3	843.336	2	7140.049	3
505	6	max	0	9	.088	3	.024	3	9.392e-3	2	NC	4	NC	1
506		min	0	3	-.137	2	-.015	2	-7.028e-3	3	837.625	2	5922.339	3
507	7	max	0	9	.085	3	.026	3	1.041e-2	2	NC	4	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.128	2	-.02	2	-7.764e-3	3	911.921	2	5264.72	3
509	8	max	0	9	.079	3	.027	3	1.142e-2	2	NC	4	NC	1
510		min	0	3	-.114	2	-.024	2	-8.5e-3	3	1068.429	2	4957.616	3
511	9	max	0	9	.072	3	.027	3	1.243e-2	2	NC	4	NC	4
512		min	0	3	-.1	2	-.028	2	-9.237e-3	3	1290.199	2	4855.378	2
513	10	max	0	9	.069	3	.027	3	1.345e-2	2	NC	4	NC	4
514		min	0	3	-.093	2	-.03	2	-9.973e-3	3	1432.247	2	4469.429	2
515	11	max	0	9	.072	3	.025	3	1.243e-2	2	NC	4	NC	4
516		min	0	3	-.1	2	-.028	2	-9.233e-3	3	1290.199	2	4855.383	2
517	12	max	0	9	.079	3	.023	3	1.142e-2	2	NC	4	NC	1
518		min	0	3	-.114	2	-.024	2	-8.494e-3	3	1068.429	2	6078.98	2
519	13	max	0	9	.085	3	.021	3	1.041e-2	2	NC	4	NC	1
520		min	0	3	-.128	2	-.02	2	-7.754e-3	3	911.921	2	7298.586	3
521	14	max	0	9	.088	3	.019	3	9.393e-3	2	NC	4	NC	1
522		min	0	3	-.137	2	-.015	2	-7.015e-3	3	837.625	2	8876.835	3
523	15	max	0	9	.086	3	.017	3	8.379e-3	2	NC	4	NC	2
524		min	0	3	-.136	2	-.012	2	-6.275e-3	3	843.336	2	8119.661	1
525	16	max	0	9	.078	3	.015	3	7.365e-3	2	NC	4	NC	2
526		min	0	3	-.124	2	-.01	2	-5.535e-3	3	951.038	2	8298.107	1
527	17	max	0	9	.064	3	.012	3	6.351e-3	2	NC	4	NC	1
528		min	0	3	-.101	2	-.009	2	-4.796e-3	3	1263.76	2	NC	1
529	18	max	0	9	.045	3	.01	3	5.337e-3	2	NC	4	NC	1
530		min	0	3	-.068	2	-.009	2	-4.056e-3	3	2344.07	2	NC	1
531	19	max	0	9	.023	3	.009	3	4.323e-3	2	NC	1	NC	1
532		min	0	3	-.03	2	-.01	2	-3.316e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	4.164e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.138e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.958e-4	3	NC	1	NC	1
536		min	0	2	-.003	4	0	3	-4.102e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	2	1.175e-3	3	NC	1	NC	1
538		min	0	2	-.005	4	-.003	3	-7.789e-4	2	NC	1	8671.965	3
539	4	max	0	3	-.002	15	.006	2	1.555e-3	3	NC	1	NC	4
540		min	0	2	-.008	4	-.007	3	-1.148e-3	2	8308.601	4	4815.173	3
541	5	max	0	3	-.002	15	.009	2	1.934e-3	3	NC	3	NC	4
542		min	0	2	-.01	4	-.012	3	-1.516e-3	2	6483.285	4	3173.748	3
543	6	max	0	3	-.003	15	.013	2	2.313e-3	3	NC	5	NC	4
544		min	-.001	2	-.011	4	-.017	3	-1.885e-3	2	5456.37	4	2317.661	3
545	7	max	0	3	-.003	15	.018	2	2.693e-3	3	NC	5	NC	4
546		min	-.001	2	-.013	4	-.022	3	-2.254e-3	2	4838.816	4	1815.668	3
547	8	max	0	3	-.003	15	.022	2	3.072e-3	3	NC	5	NC	4
548		min	-.002	2	-.014	4	-.028	3	-2.623e-3	2	4468.19	4	1499.492	3
549	9	max	0	3	-.003	15	.025	2	3.452e-3	3	NC	5	NC	4
550		min	-.002	2	-.015	4	-.032	3	-2.992e-3	2	4268.698	4	1292.335	3
551	10	max	.001	3	-.003	15	.028	2	3.831e-3	3	NC	5	NC	4
552		min	-.002	2	-.015	4	-.036	3	-3.36e-3	2	4205.588	4	1155.541	3
553	11	max	.001	3	-.003	15	.03	2	4.21e-3	3	NC	5	NC	4
554		min	-.002	2	-.015	4	-.039	3	-3.729e-3	2	4268.698	4	1068.799	3
555	12	max	.001	3	-.003	15	.03	2	4.59e-3	3	NC	5	NC	4
556		min	-.003	2	-.014	4	-.039	3	-4.098e-3	2	4468.19	4	1022.049	3
557	13	max	.001	3	-.003	15	.029	2	4.969e-3	3	NC	5	NC	4
558		min	-.003	2	-.013	4	-.038	3	-4.467e-3	2	4838.816	4	1012.434	3
559	14	max	.002	3	-.002	2	.025	2	5.349e-3	3	NC	5	NC	4
560		min	-.003	2	-.012	4	-.034	3	-4.835e-3	2	5456.37	4	1044.456	3
561	15	max	.002	3	0	2	.02	2	5.728e-3	3	NC	3	NC	4
562		min	-.003	2	-.01	4	-.027	3	-5.204e-3	2	6483.285	4	1134.416	3
563	16	max	.002	3	.002	2	.013	1	6.107e-3	3	NC	1	NC	4
564		min	-.004	2	-.008	4	-.018	3	-5.573e-3	2	8308.601	4	1326.483	3

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.002	3	.005	2	.004	1	6.487e-3	3	NC	1	NC	4
566			min	-.004	2	-.005	4	-.004	3	-5.942e-3	2	NC	1	1759.154	3
567		18	max	.002	3	.007	2	.013	3	6.866e-3	3	NC	1	NC	4
568			min	-.004	2	-.003	4	-.014	2	-6.311e-3	2	NC	1	3132.962	3
569		19	max	.002	3	.01	2	.035	3	7.246e-3	3	NC	1	NC	1
570			min	-.004	2	-.002	3	-.032	2	-6.679e-3	2	NC	1	NC	1
571	M16A	1	max	.001	2	.003	2	.011	3	2.03e-3	3	NC	1	NC	1
572			min	-.002	3	-.003	3	-.01	2	-2.122e-3	2	NC	1	NC	1
573		2	max	.001	2	0	2	.003	3	1.958e-3	3	NC	1	NC	1
574			min	-.002	3	-.004	3	-.005	2	-2.026e-3	2	NC	1	8722.988	3
575		3	max	.001	2	-.001	15	.002	1	1.887e-3	3	NC	1	NC	4
576			min	-.002	3	-.006	3	-.003	3	-1.93e-3	2	NC	1	4943.826	3
577		4	max	0	2	-.002	15	.005	1	1.815e-3	3	NC	1	NC	4
578			min	-.002	3	-.008	4	-.008	3	-1.833e-3	2	8308.601	4	3767.42	3
579		5	max	0	2	-.002	15	.007	1	1.743e-3	3	NC	3	NC	4
580			min	-.002	3	-.01	4	-.011	3	-1.737e-3	2	6483.285	4	3260.963	3
581		6	max	0	2	-.003	15	.008	1	1.672e-3	3	NC	5	NC	4
582			min	-.002	3	-.012	4	-.013	3	-1.641e-3	2	5456.37	4	3044.322	3
583		7	max	0	2	-.003	15	.009	1	1.6e-3	3	NC	5	NC	4
584			min	-.001	3	-.013	4	-.014	3	-1.544e-3	2	4838.816	4	2999.078	3
585		8	max	0	2	-.003	15	.009	1	1.528e-3	3	NC	5	NC	4
586			min	-.001	3	-.014	4	-.014	3	-1.448e-3	2	4468.19	4	3085.856	3
587		9	max	0	2	-.003	15	.008	1	1.457e-3	3	NC	5	NC	4
588			min	-.001	3	-.015	4	-.013	3	-1.352e-3	2	4268.698	4	3301.576	3
589		10	max	0	2	-.003	15	.008	1	1.385e-3	3	NC	5	NC	4
590			min	-.001	3	-.015	4	-.012	3	-1.255e-3	2	4205.588	4	3670.399	3
591		11	max	0	2	-.003	15	.007	1	1.313e-3	3	NC	5	NC	4
592			min	0	3	-.015	4	-.01	3	-1.159e-3	2	4268.698	4	4250.126	3
593		12	max	0	2	-.003	15	.005	1	1.242e-3	3	NC	5	NC	4
594			min	0	3	-.014	4	-.008	3	-1.063e-3	2	4468.19	4	5156.445	3
595		13	max	0	2	-.003	15	.004	1	1.17e-3	3	NC	5	NC	1
596			min	0	3	-.013	4	-.006	3	-9.663e-4	2	4838.816	4	6626.039	3
597		14	max	0	2	-.003	15	.003	1	1.098e-3	3	NC	5	NC	1
598			min	0	3	-.011	4	-.004	3	-8.7e-4	2	5456.37	4	9192.438	3
599		15	max	0	2	-.002	15	.002	1	1.027e-3	3	NC	3	NC	1
600			min	0	3	-.01	4	-.002	3	-7.736e-4	2	6483.285	4	NC	1
601		16	max	0	2	-.002	15	.001	4	9.551e-4	3	NC	1	NC	1
602			min	0	3	-.008	4	0	3	-6.773e-4	2	8308.601	4	NC	1
603		17	max	0	2	-.001	15	0	4	8.834e-4	3	NC	1	NC	1
604			min	0	3	-.005	4	0	2	-5.81e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	3	8.118e-4	3	NC	1	NC	1
606			min	0	3	-.003	4	0	2	-4.847e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	7.401e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.883e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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Address:			
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5}$  (Eq. D-7)

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$  (Sec. D.4.1 & Eq. D-5)

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$  (Eq. D-16f)

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$  (Sec. D.4.1 & Eq. D-16b)

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

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