



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.050 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.789 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	21%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.902 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<u>20%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.056 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>14%</u>



A cross brace kit is required every 14 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

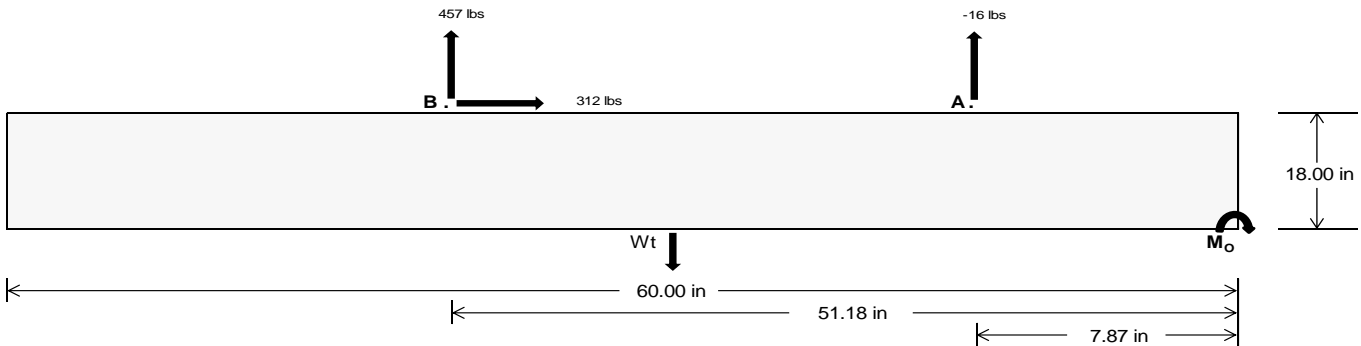
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>35.97</u>	<u>1903.94</u>	k
Compressive Load =	<u>1364.88</u>	<u>1415.96</u>	k
Lateral Load =	<u>5.58</u>	<u>1298.80</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 28874.0$ in-lbs
Resisting Force Required = 962.47 lbs
S.F. = 1.67
Weight Required = 1604.11 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 312.03 lbs
Friction = 0.4
Weight Required = 780.08 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 312.03 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	548 lbs	548 lbs	548 lbs	548 lbs	381 lbs	381 lbs	381 lbs	381 lbs	642 lbs	642 lbs	642 lbs	642 lbs	32 lbs	32 lbs	32 lbs	32 lbs
F_B	372 lbs	372 lbs	372 lbs	372 lbs	607 lbs	607 lbs	607 lbs	607 lbs	696 lbs	696 lbs	696 lbs	696 lbs	-914 lbs	-914 lbs	-914 lbs	-914 lbs
F_V	74 lbs	74 lbs	74 lbs	74 lbs	574 lbs	574 lbs	574 lbs	574 lbs	479 lbs	479 lbs	479 lbs	479 lbs	-624 lbs	-624 lbs	-624 lbs	-624 lbs
P_{total}	2914 lbs	3005 lbs	3096 lbs	3186 lbs	2982 lbs	3072 lbs	3163 lbs	3253 lbs	3332 lbs	3423 lbs	3513 lbs	3604 lbs	315 lbs	369 lbs	424 lbs	478 lbs
M	466 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	491 lbs-ft	491 lbs-ft	491 lbs-ft	491 lbs-ft	674 lbs-ft	674 lbs-ft	674 lbs-ft	674 lbs-ft	736 lbs-ft	736 lbs-ft	736 lbs-ft	736 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.16 ft	0.16 ft	0.16 ft	0.15 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.34 ft	2.00 ft	1.74 ft	1.54 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	257.0 psf	255.3 psf	253.7 psf	252.2 psf	261.0 psf	259.1 psf	257.4 psf	255.8 psf	275.3 psf	272.8 psf	270.5 psf	268.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	378.9 psf	371.9 psf	365.4 psf	359.5 psf	389.5 psf	382.0 psf	375.2 psf	368.9 psf	451.7 psf	441.5 psf	432.2 psf	423.6 psf	713.8 psf	254.3 psf	185.5 psf	159.5 psf

Maximum Bearing Pressure = 714 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

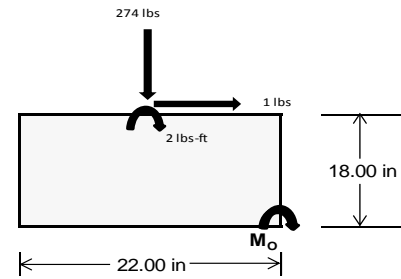
Overturning Check

$M_o = 247.6 \text{ ft-lbs}$
 Resisting Force Required = 270.10 lbs
 S.F. = 1.67
 Weight Required = 450.17 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	91 lbs	230 lbs	86 lbs	279 lbs	770 lbs	274 lbs	27 lbs	67 lbs	25 lbs
F_v	7 lbs	6 lbs	0 lbs	24 lbs	22 lbs	1 lbs	2 lbs	2 lbs	0 lbs
P_{total}	2559 lbs	2698 lbs	2555 lbs	2629 lbs	3120 lbs	2624 lbs	748 lbs	789 lbs	747 lbs
M	11 lbs-ft	9 lbs-ft	1 lbs-ft	41 lbs-ft	34 lbs-ft	4 lbs-ft	3 lbs-ft	3 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.02 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.80 ft	1.81 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	275.4 sqft	291.1 sqft	278.5 sqft	272.3 sqft	328.3 sqft	284.8 sqft	80.6 sqft	85.1 sqft	81.4 sqft
f_{max}	283.1 psf	297.6 psf	278.9 psf	301.3 psf	352.4 psf	287.7 psf	82.7 psf	87.0 psf	81.5 psf



Maximum Bearing Pressure = 352 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.403 k
Allowable Uplift =	1.214 k
Utilization =	<u>33%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.107 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.050 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>18%</u>

Diagonal Strut

Maximum Axial Load =	0.789 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.197 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.056 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.083 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.255$$

$$242.167$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.255$$

$$251.476$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.2$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.3 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.203 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max St} = 1.463 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max Wk} = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

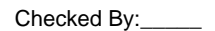
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	249.347	2	315.934	2	-.005	15	0	15	0	1	0	1
2		min	-310.537	3	-444.603	3	-.205	1	0	1	0	1	0	1
3	N7	max	.028	3	425.548	1	-.105	15	0	15	0	1	0	1
4		min	-.182	2	17.884	15	-2.043	1	-.004	1	0	1	0	1
5	N15	max	.225	3	1049.908	1	.702	1	.001	1	0	1	0	1
6		min	-1.815	2	39.43	15	-.439	3	0	3	0	1	0	1
7	N16	max	942.904	2	1089.201	2	-.291	10	0	1	0	1	0	1
8		min	-999.079	3	-1464.568	3	-50.004	3	0	3	0	1	0	1
9	N23	max	.028	3	425.213	1	4.289	1	.007	1	0	1	0	1
10		min	-.182	2	18.051	15	.209	15	0	15	0	1	0	1
11	N24	max	249.924	2	320.575	2	50.314	3	.002	1	0	1	0	1
12		min	-310.659	3	-441.954	3	.029	10	0	12	0	1	0	1
13	Totals:	max	1439.994	2	3419.693	1	0	11						
14		min	-1619.994	3	-2126.537	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	275.719	1	.677	4	.571	1	0	15	0	12	0	1
2			min	-366.895	3	.159	15	-.025	3	-.001	1	0	1	0	1
3		2	max	275.853	1	.62	4	.571	1	0	15	0	12	0	15
4			min	-366.794	3	.146	15	-.025	3	-.001	1	0	1	0	4
5		3	max	275.988	1	.562	4	.571	1	0	15	0	15	0	15
6			min	-366.693	3	.132	15	-.025	3	-.001	1	0	1	0	4
7		4	max	276.123	1	.505	4	.571	1	0	15	0	15	0	15
8			min	-366.592	3	.119	15	-.025	3	-.001	1	0	1	0	4
9		5	max	276.258	1	.447	4	.571	1	0	15	0	1	0	15
10			min	-366.491	3	.105	15	-.025	3	-.001	1	0	3	0	4
11		6	max	276.393	1	.39	4	.571	1	0	15	0	1	0	15
12			min	-366.39	3	.092	15	-.025	3	-.001	1	0	3	0	4
13		7	max	276.528	1	.332	4	.571	1	0	15	0	1	0	15
14			min	-366.288	3	.078	15	-.025	3	-.001	1	0	3	0	4
15		8	max	276.663	1	.275	4	.571	1	0	15	0	1	0	15
16			min	-366.187	3	.065	15	-.025	3	-.001	1	0	3	0	4
17		9	max	276.797	1	.217	4	.571	1	0	15	0	1	0	15
18			min	-366.086	3	.051	15	-.025	3	-.001	1	0	3	0	4
19		10	max	276.932	1	.16	4	.571	1	0	15	0	1	0	15
20			min	-365.985	3	.037	12	-.025	3	-.001	1	0	3	0	4
21		11	max	277.067	1	.108	2	.571	1	0	15	0	1	0	15
22			min	-365.884	3	.015	12	-.025	3	-.001	1	0	3	0	4
23		12	max	277.202	1	.063	2	.571	1	0	15	0	1	0	15
24			min	-365.783	3	-.013	3	-.025	3	-.001	1	0	3	0	4
25		13	max	277.337	1	.018	2	.571	1	0	15	0	1	0	15
26			min	-365.682	3	-.047	3	-.025	3	-.001	1	0	3	0	4
27		14	max	277.472	1	-.016	15	.571	1	0	15	.001	1	0	15
28			min	-365.58	3	-.081	3	-.025	3	-.001	1	0	3	0	4
29		15	max	277.607	1	-.03	15	.571	1	0	15	.001	1	0	15
30			min	-365.479	3	-.128	4	-.025	3	-.001	1	0	3	0	4
31		16	max	277.742	1	-.043	15	.571	1	0	15	.001	1	0	15
32			min	-365.378	3	-.185	4	-.025	3	-.001	1	0	3	0	4
33		17	max	277.876	1	-.057	15	.571	1	0	15	.001	1	0	15
34			min	-365.277	3	-.243	4	-.025	3	-.001	1	0	3	0	4
35		18	max	278.011	1	-.07	15	.571	1	0	15	.001	1	0	15
36			min	-365.176	3	-.3	4	-.025	3	-.001	1	0	3	0	4
37		19	max	278.146	1	-.084	15	.571	1	0	15	.002	1	0	12

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-365.075	3	-.358	4	-.025	3	-.001	1	0	3	0	4
39	M3	1	max	196.944	2	1.735	4	-.03	15	0	15	.002	1	0	4
40			min	-218.474	3	.408	15	-.661	1	0	1	0	15	0	12
41		2	max	196.874	2	1.558	4	-.03	15	0	15	.002	1	0	2
42			min	-218.526	3	.366	15	-.661	1	0	1	0	15	0	3
43		3	max	196.804	2	1.382	4	-.03	15	0	15	.002	1	0	2
44			min	-218.579	3	.325	15	-.661	1	0	1	0	15	0	3
45		4	max	196.734	2	1.205	4	-.03	15	0	15	.002	1	0	15
46			min	-218.631	3	.284	15	-.661	1	0	1	0	15	0	4
47		5	max	196.664	2	1.029	4	-.03	15	0	15	.002	1	0	15
48			min	-218.684	3	.242	15	-.661	1	0	1	0	15	0	4
49		6	max	196.594	2	.853	4	-.03	15	0	15	.002	1	0	15
50			min	-218.736	3	.201	15	-.661	1	0	1	0	15	0	4
51		7	max	196.524	2	.676	4	-.03	15	0	15	.001	1	0	15
52			min	-218.789	3	.159	15	-.661	1	0	1	0	15	0	4
53		8	max	196.454	2	.5	4	-.03	15	0	15	.001	1	0	15
54			min	-218.841	3	.118	15	-.661	1	0	1	0	15	-.001	4
55		9	max	196.384	2	.324	4	-.03	15	0	15	.001	1	0	15
56			min	-218.894	3	.076	15	-.661	1	0	1	0	15	-.001	4
57		10	max	196.314	2	.147	4	-.03	15	0	15	0	1	0	15
58			min	-218.946	3	.035	12	-.661	1	0	1	0	15	-.001	4
59		11	max	196.244	2	.003	2	-.03	15	0	15	0	1	0	15
60			min	-218.999	3	-.054	3	-.661	1	0	1	0	15	-.001	4
61		12	max	196.174	2	-.048	15	-.03	15	0	15	0	1	0	15
62			min	-219.051	3	-.206	4	-.661	1	0	1	0	15	-.001	4
63		13	max	196.104	2	-.09	15	-.03	15	0	15	0	1	0	15
64			min	-219.104	3	-.382	4	-.661	1	0	1	0	15	-.001	4
65		14	max	196.034	2	-.131	15	-.03	15	0	15	0	1	0	15
66			min	-219.156	3	-.558	4	-.661	1	0	1	0	15	-.001	4
67		15	max	195.964	2	-.173	15	-.03	15	0	15	0	1	0	15
68			min	-219.209	3	-.735	4	-.661	1	0	1	0	12	0	4
69		16	max	195.894	2	-.214	15	-.03	15	0	15	0	1	0	15
70			min	-219.261	3	-.911	4	-.661	1	0	1	0	12	0	4
71		17	max	195.824	2	-.255	15	-.03	15	0	15	0	15	0	15
72			min	-219.314	3	-1.087	4	-.661	1	0	1	0	1	0	4
73		18	max	195.754	2	-.297	15	-.03	15	0	15	0	15	0	15
74			min	-219.366	3	-1.264	4	-.661	1	0	1	0	1	0	4
75		19	max	195.684	2	-.338	15	-.03	15	0	15	0	15	0	1
76			min	-219.419	3	-1.44	4	-.661	1	0	1	0	1	0	1
77	M4	1	max	424.383	1	0	1	-.105	15	0	1	0	3	0	1
78			min	17.533	15	0	1	-2.199	1	0	1	0	2	0	1
79		2	max	424.448	1	0	1	-.105	15	0	1	0	12	0	1
80			min	17.552	15	0	1	-2.199	1	0	1	0	1	0	1
81		3	max	424.512	1	0	1	-.105	15	0	1	0	15	0	1
82			min	17.572	15	0	1	-2.199	1	0	1	0	1	0	1
83		4	max	424.577	1	0	1	-.105	15	0	1	0	15	0	1
84			min	17.591	15	0	1	-2.199	1	0	1	0	1	0	1
85		5	max	424.642	1	0	1	-.105	15	0	1	0	15	0	1
86			min	17.611	15	0	1	-2.199	1	0	1	0	1	0	1
87		6	max	424.707	1	0	1	-.105	15	0	1	0	15	0	1
88			min	17.63	15	0	1	-2.199	1	0	1	-.001	1	0	1
89		7	max	424.771	1	0	1	-.105	15	0	1	0	15	0	1
90			min	17.65	15	0	1	-2.199	1	0	1	-.001	1	0	1
91		8	max	424.836	1	0	1	-.105	15	0	1	0	15	0	1
92			min	17.669	15	0	1	-2.199	1	0	1	-.001	1	0	1
93		9	max	424.901	1	0	1	-.105	15	0	1	0	15	0	1
94			min	17.689	15	0	1	-2.199	1	0	1	-.002	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	424.965	1	0	1	-.105	15	0	1	0	15	0	1
96		min	17.708	15	0	1	-2.199	1	0	1	-.002	1	0	1
97	11	max	425.03	1	0	1	-.105	15	0	1	0	15	0	1
98		min	17.728	15	0	1	-2.199	1	0	1	-.002	1	0	1
99	12	max	425.095	1	0	1	-.105	15	0	1	0	15	0	1
100		min	17.747	15	0	1	-2.199	1	0	1	-.002	1	0	1
101	13	max	425.16	1	0	1	-.105	15	0	1	0	15	0	1
102		min	17.767	15	0	1	-2.199	1	0	1	-.002	1	0	1
103	14	max	425.224	1	0	1	-.105	15	0	1	0	15	0	1
104		min	17.786	15	0	1	-2.199	1	0	1	-.003	1	0	1
105	15	max	425.289	1	0	1	-.105	15	0	1	0	15	0	1
106		min	17.806	15	0	1	-2.199	1	0	1	-.003	1	0	1
107	16	max	425.354	1	0	1	-.105	15	0	1	0	15	0	1
108		min	17.825	15	0	1	-2.199	1	0	1	-.003	1	0	1
109	17	max	425.418	1	0	1	-.105	15	0	1	0	15	0	1
110		min	17.845	15	0	1	-2.199	1	0	1	-.003	1	0	1
111	18	max	425.483	1	0	1	-.105	15	0	1	0	15	0	1
112		min	17.864	15	0	1	-2.199	1	0	1	-.003	1	0	1
113	19	max	425.548	1	0	1	-.105	15	0	1	0	15	0	1
114		min	17.884	15	0	1	-2.199	1	0	1	-.004	1	0	1
115	M6	1	max	899.369	1	.682	4	.138	1	0	0	3	0	1
116		min	-1196.615	3	.16	15	-.137	3	0	15	0	11	0	1
117	2	max	899.504	1	.625	4	.138	1	0	1	0	3	0	15
118		min	-1196.514	3	.147	15	-.137	3	0	15	0	11	0	4
119	3	max	899.639	1	.567	4	.138	1	0	1	0	3	0	15
120		min	-1196.412	3	.133	15	-.137	3	0	15	0	15	0	4
121	4	max	899.774	1	.51	4	.138	1	0	1	0	3	0	15
122		min	-1196.311	3	.118	12	-.137	3	0	15	0	15	0	4
123	5	max	899.909	1	.452	4	.138	1	0	1	0	1	0	15
124		min	-1196.21	3	.096	12	-.137	3	0	15	0	15	0	4
125	6	max	900.044	1	.404	2	.138	1	0	1	0	1	0	15
126		min	-1196.109	3	.073	12	-.137	3	0	15	0	15	0	4
127	7	max	900.179	1	.36	2	.138	1	0	1	0	1	0	12
128		min	-1196.008	3	.051	12	-.137	3	0	15	0	12	0	4
129	8	max	900.313	1	.315	2	.138	1	0	1	0	1	0	12
130		min	-1195.907	3	.028	12	-.137	3	0	15	0	3	0	4
131	9	max	900.448	1	.27	2	.138	1	0	1	0	1	0	12
132		min	-1195.806	3	0	3	-.137	3	0	15	0	3	0	4
133	10	max	900.583	1	.225	2	.138	1	0	1	0	1	0	12
134		min	-1195.704	3	-.033	3	-.137	3	0	15	0	3	0	2
135	11	max	900.718	1	.18	2	.138	1	0	1	0	1	0	12
136		min	-1195.603	3	-.066	3	-.137	3	0	15	0	3	0	2
137	12	max	900.853	1	.136	2	.138	1	0	1	0	1	0	12
138		min	-1195.502	3	-.1	3	-.137	3	0	15	0	3	0	2
139	13	max	900.988	1	.091	2	.138	1	0	1	0	1	0	12
140		min	-1195.401	3	-.133	3	-.137	3	0	15	0	3	0	2
141	14	max	901.123	1	.046	2	.138	1	0	1	0	1	0	12
142		min	-1195.3	3	-.167	3	-.137	3	0	15	0	3	0	2
143	15	max	901.258	1	.001	2	.138	1	0	1	0	1	0	12
144		min	-1195.199	3	-.201	3	-.137	3	0	15	0	3	0	2
145	16	max	901.392	1	-.043	15	.138	1	0	1	0	1	0	12
146		min	-1195.097	3	-.234	3	-.137	3	0	15	0	3	0	2
147	17	max	901.527	1	-.056	15	.138	1	0	1	0	1	0	3
148		min	-1194.996	3	-.268	3	-.137	3	0	15	0	3	0	2
149	18	max	901.662	1	-.07	15	.138	1	0	1	0	1	0	3
150		min	-1194.895	3	-.301	3	-.137	3	0	15	0	3	0	2
151	19	max	901.797	1	-.083	15	.138	1	0	1	0	1	0	3



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152			min	-1194.794	3	-.352	4	-.137	3	0	15	0	3	0	2
153	M7	1	max	789.336	2	1.741	4	.025	3	0	2	0	2	0	2
154			min	-688.454	3	.409	15	-.004	10	0	3	0	3	0	3
155		2	max	789.266	2	1.564	4	.025	3	0	2	0	2	0	2
156			min	-688.507	3	.367	15	-.004	10	0	3	0	3	0	3
157		3	max	789.196	2	1.388	4	.025	3	0	2	0	2	0	2
158			min	-688.559	3	.326	15	-.004	10	0	3	0	3	0	3
159		4	max	789.126	2	1.212	4	.025	3	0	2	0	2	0	2
160			min	-688.612	3	.285	15	-.004	10	0	3	0	3	0	3
161		5	max	789.056	2	1.035	4	.025	3	0	2	0	2	0	15
162			min	-688.664	3	.243	15	-.004	10	0	3	0	3	0	3
163		6	max	788.986	2	.859	4	.025	3	0	2	0	2	0	15
164			min	-688.717	3	.202	15	-.004	10	0	3	0	3	0	3
165		7	max	788.916	2	.682	4	.025	3	0	2	0	2	0	15
166			min	-688.769	3	.16	15	-.004	10	0	3	0	3	0	4
167		8	max	788.846	2	.506	4	.025	3	0	2	0	2	0	15
168			min	-688.822	3	.118	12	-.004	10	0	3	0	3	-.001	4
169		9	max	788.776	2	.351	2	.025	3	0	2	0	2	0	15
170			min	-688.874	3	.049	12	-.004	10	0	3	0	3	-.001	4
171		10	max	788.706	2	.214	2	.025	3	0	2	0	2	0	15
172			min	-688.927	3	-.037	3	-.004	10	0	3	0	3	-.001	4
173		11	max	788.636	2	.076	2	.025	3	0	2	0	2	0	15
174			min	-688.979	3	-.14	3	-.004	10	0	3	0	3	-.001	4
175		12	max	788.566	2	-.047	15	.025	3	0	2	0	2	0	15
176			min	-689.032	3	-.243	3	-.004	10	0	3	0	3	-.001	4
177		13	max	788.496	2	-.089	15	.025	3	0	2	0	2	0	15
178			min	-689.084	3	-.376	4	-.004	10	0	3	0	3	-.001	4
179		14	max	788.426	2	-.13	15	.025	3	0	2	0	2	0	15
180			min	-689.137	3	-.552	4	-.004	10	0	3	0	3	-.001	4
181		15	max	788.356	2	-.172	15	.025	3	0	2	0	2	0	15
182			min	-689.189	3	-.729	4	-.004	10	0	3	0	3	0	4
183		16	max	788.286	2	-.213	15	.025	3	0	2	0	2	0	15
184			min	-689.242	3	-.905	4	-.004	10	0	3	0	3	0	4
185		17	max	788.216	2	-.254	15	.025	3	0	2	0	2	0	15
186			min	-689.294	3	-1.081	4	-.004	10	0	3	0	3	0	4
187		18	max	788.146	2	-.296	15	.025	3	0	2	0	2	0	15
188			min	-689.347	3	-1.258	4	-.004	10	0	3	0	3	0	4
189		19	max	788.076	2	-.337	15	.025	3	0	2	0	2	0	1
190			min	-689.399	3	-1.434	4	-.004	10	0	3	0	3	0	1
191	M8	1	max	1048.743	1	0	1	.846	1	0	1	0	15	0	1
192			min	39.078	15	0	1	-.448	3	0	1	0	1	0	1
193		2	max	1048.808	1	0	1	.846	1	0	1	0	1	0	1
194			min	39.098	15	0	1	-.448	3	0	1	0	3	0	1
195		3	max	1048.872	1	0	1	.846	1	0	1	0	1	0	1
196			min	39.117	15	0	1	-.448	3	0	1	0	3	0	1
197		4	max	1048.937	1	0	1	.846	1	0	1	0	1	0	1
198			min	39.137	15	0	1	-.448	3	0	1	0	3	0	1
199		5	max	1049.002	1	0	1	.846	1	0	1	0	1	0	1
200			min	39.156	15	0	1	-.448	3	0	1	0	3	0	1
201		6	max	1049.067	1	0	1	.846	1	0	1	0	1	0	1
202			min	39.176	15	0	1	-.448	3	0	1	0	3	0	1
203		7	max	1049.131	1	0	1	.846	1	0	1	0	1	0	1
204			min	39.195	15	0	1	-.448	3	0	1	0	3	0	1
205		8	max	1049.196	1	0	1	.846	1	0	1	0	1	0	1
206			min	39.215	15	0	1	-.448	3	0	1	0	3	0	1
207		9	max	1049.261	1	0	1	.846	1	0	1	0	1	0	1
208			min	39.234	15	0	1	-.448	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	1049.325	1	0	1	.846	1	0	1	0	1	0	1
210		min	39.254	15	0	1	-.448	3	0	1	0	3	0	1
211	11	max	1049.39	1	0	1	.846	1	0	1	0	1	0	1
212		min	39.273	15	0	1	-.448	3	0	1	0	3	0	1
213	12	max	1049.455	1	0	1	.846	1	0	1	0	1	0	1
214		min	39.293	15	0	1	-.448	3	0	1	0	3	0	1
215	13	max	1049.52	1	0	1	.846	1	0	1	0	1	0	1
216		min	39.312	15	0	1	-.448	3	0	1	0	3	0	1
217	14	max	1049.584	1	0	1	.846	1	0	1	0	1	0	1
218		min	39.332	15	0	1	-.448	3	0	1	0	3	0	1
219	15	max	1049.649	1	0	1	.846	1	0	1	.001	1	0	1
220		min	39.351	15	0	1	-.448	3	0	1	0	3	0	1
221	16	max	1049.714	1	0	1	.846	1	0	1	.001	1	0	1
222		min	39.371	15	0	1	-.448	3	0	1	0	3	0	1
223	17	max	1049.778	1	0	1	.846	1	0	1	.001	1	0	1
224		min	39.391	15	0	1	-.448	3	0	1	0	3	0	1
225	18	max	1049.843	1	0	1	.846	1	0	1	.001	1	0	1
226		min	39.41	15	0	1	-.448	3	0	1	0	3	0	1
227	19	max	1049.908	1	0	1	.846	1	0	1	.001	1	0	1
228		min	39.43	15	0	1	-.448	3	0	1	0	3	0	1
229	M10	1	max	292.469	1	.672	4	.005	3	.001	1	0	1	1
230		min	-342.624	3	.159	15	-.246	1	0	3	0	3	0	1
231	2	max	292.603	1	.615	4	.005	3	.001	1	0	1	0	15
232		min	-342.523	3	.145	15	-.246	1	0	3	0	3	0	4
233	3	max	292.738	1	.557	4	.005	3	.001	1	0	1	0	15
234		min	-342.421	3	.132	15	-.246	1	0	3	0	3	0	4
235	4	max	292.873	1	.5	4	.005	3	.001	1	0	1	0	15
236		min	-342.32	3	.118	15	-.246	1	0	3	0	3	0	4
237	5	max	293.008	1	.443	4	.005	3	.001	1	0	1	0	15
238		min	-342.219	3	.105	15	-.246	1	0	3	0	3	0	4
239	6	max	293.143	1	.385	4	.005	3	.001	1	0	1	0	15
240		min	-342.118	3	.091	15	-.246	1	0	3	0	3	0	4
241	7	max	293.278	1	.328	4	.005	3	.001	1	0	1	0	15
242		min	-342.017	3	.078	15	-.246	1	0	3	0	3	0	4
243	8	max	293.413	1	.27	4	.005	3	.001	1	0	1	0	15
244		min	-341.916	3	.064	15	-.246	1	0	3	0	3	0	4
245	9	max	293.548	1	.213	4	.005	3	.001	1	0	1	0	15
246		min	-341.815	3	.05	15	-.246	1	0	3	0	3	0	4
247	10	max	293.682	1	.155	4	.005	3	.001	1	0	1	0	15
248		min	-341.713	3	.037	15	-.246	1	0	3	0	3	0	4
249	11	max	293.817	1	.108	2	.005	3	.001	1	0	11	0	15
250		min	-341.612	3	.023	15	-.246	1	0	3	0	3	0	4
251	12	max	293.952	1	.063	2	.005	3	.001	1	0	15	0	15
252		min	-341.511	3	.007	12	-.246	1	0	3	0	3	0	4
253	13	max	294.087	1	.019	2	.005	3	.001	1	0	15	0	15
254		min	-341.41	3	-.024	1	-.246	1	0	3	0	3	0	4
255	14	max	294.222	1	-.017	15	.005	3	.001	1	0	15	0	15
256		min	-341.309	3	-.075	4	-.246	1	0	3	0	1	0	4
257	15	max	294.357	1	-.031	15	.005	3	.001	1	0	15	0	15
258		min	-341.208	3	-.132	4	-.246	1	0	3	0	1	0	4
259	16	max	294.492	1	-.044	15	.005	3	.001	1	0	15	0	15
260		min	-341.107	3	-.19	4	-.246	1	0	3	0	1	0	4
261	17	max	294.626	1	-.058	15	.005	3	.001	1	0	15	0	15
262		min	-341.005	3	-.247	4	-.246	1	0	3	0	1	0	4
263	18	max	294.761	1	-.071	15	.005	3	.001	1	0	15	0	15
264		min	-340.904	3	-.305	4	-.246	1	0	3	0	1	0	4
265	19	max	294.896	1	-.085	15	.005	3	.001	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-340.803	3	-.362	4	-.246	1	0	3	0	1	0	4
267		1	max	196.674	2	1.739	4	.726	1	.001	1	0	3	0	4
268			min	-219.136	3	.409	15	0	3	0	15	-.002	1	0	12
269		2	max	196.604	2	1.563	4	.726	1	.001	1	0	3	0	1
270			min	-219.188	3	.367	15	0	3	0	15	-.002	1	0	3
271		3	max	196.534	2	1.386	4	.726	1	.001	1	0	3	0	1
272			min	-219.241	3	.326	15	0	3	0	15	-.002	1	0	3
273		4	max	196.464	2	1.21	4	.726	1	.001	1	0	3	0	15
274			min	-219.293	3	.284	15	0	3	0	15	-.002	1	0	3
275		5	max	196.394	2	1.034	4	.726	1	.001	1	0	3	0	15
276			min	-219.346	3	.243	15	0	3	0	15	-.002	1	0	4
277		6	max	196.324	2	.857	4	.726	1	.001	1	0	3	0	15
278			min	-219.398	3	.201	15	0	3	0	15	-.001	1	0	4
279		7	max	196.254	2	.681	4	.726	1	.001	1	0	3	0	15
280			min	-219.451	3	.16	15	0	3	0	15	-.001	1	0	4
281		8	max	196.184	2	.504	4	.726	1	.001	1	0	3	0	15
282			min	-219.503	3	.118	15	0	3	0	15	-.001	1	-.001	4
283	9	max	196.114	2	.328	4	.726	1	.001	1	0	3	0	15	
284		min	-219.556	3	.077	15	0	3	0	15	0	1	-.001	4	
285	10	max	196.044	2	.152	4	.726	1	.001	1	0	3	0	15	
286		min	-219.608	3	.023	12	0	3	0	15	0	1	-.001	4	
287	11	max	195.974	2	.006	1	.726	1	.001	1	0	3	0	15	
288		min	-219.661	3	-.072	3	0	3	0	15	0	1	-.001	4	
289	12	max	195.904	2	-.047	15	.726	1	.001	1	0	3	0	15	
290		min	-219.713	3	-.201	4	0	3	0	15	0	1	-.001	4	
291	13	max	195.834	2	-.089	15	.726	1	.001	1	0	3	0	15	
292		min	-219.766	3	-.377	4	0	3	0	15	0	1	-.001	4	
293	14	max	195.764	2	-.13	15	.726	1	.001	1	0	3	0	15	
294		min	-219.818	3	-.554	4	0	3	0	15	0	1	-.001	4	
295	15	max	195.694	2	-.172	15	.726	1	.001	1	0	3	0	15	
296		min	-219.871	3	-.73	4	0	3	0	15	0	2	0	4	
297	16	max	195.624	2	-.213	15	.726	1	.001	1	0	3	0	15	
298		min	-219.923	3	-.907	4	0	3	0	15	0	10	0	4	
299	17	max	195.554	2	-.255	15	.726	1	.001	1	0	1	0	15	
300		min	-219.976	3	-1.083	4	0	3	0	15	0	15	0	4	
301	18	max	195.484	2	-.296	15	.726	1	.001	1	0	1	0	15	
302		min	-220.028	3	-1.259	4	0	3	0	15	0	15	0	4	
303	19	max	195.414	2	-.338	15	.726	1	.001	1	0	1	0	1	
304		min	-220.081	3	-1.436	4	0	3	0	15	0	15	0	1	
305	M12	1	max	424.048	1	0	1	4.612	1	0	1	0	2	0	1
306		min	17.699	15	0	1	.209	15	0	1	0	3	0	1	
307	2	max	424.113	1	0	1	4.612	1	0	1	0	1	0	1	
308		min	17.719	15	0	1	.209	15	0	1	0	12	0	1	
309	3	max	424.178	1	0	1	4.612	1	0	1	0	1	0	1	
310		min	17.738	15	0	1	.209	15	0	1	0	15	0	1	
311	4	max	424.242	1	0	1	4.612	1	0	1	.001	1	0	1	
312		min	17.758	15	0	1	.209	15	0	1	0	15	0	1	
313	5	max	424.307	1	0	1	4.612	1	0	1	.002	1	0	1	
314		min	17.777	15	0	1	.209	15	0	1	0	15	0	1	
315	6	max	424.372	1	0	1	4.612	1	0	1	.002	1	0	1	
316		min	17.797	15	0	1	.209	15	0	1	0	15	0	1	
317	7	max	424.436	1	0	1	4.612	1	0	1	.003	1	0	1	
318		min	17.816	15	0	1	.209	15	0	1	0	15	0	1	
319	8	max	424.501	1	0	1	4.612	1	0	1	.003	1	0	1	
320		min	17.836	15	0	1	.209	15	0	1	0	15	0	1	
321	9	max	424.566	1	0	1	4.612	1	0	1	.003	1	0	1	
322		min	17.855	15	0	1	.209	15	0	1	0	15	0	1	







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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	118.584	3	6.015	9	87.247	1	0	1	.008	3	.153	2
438		min	-12.661	10	-29.658	2	1.882	12	0	12	0	2	-.133	3
439	11	max	118.704	3	5.824	9	87.247	1	0	1	.023	1	.159	2
440		min	-12.527	10	-29.886	2	1.882	12	0	12	.001	15	-.13	3
441	12	max	118.824	3	5.634	9	87.247	1	0	1	.042	1	.166	2
442		min	-12.394	10	-30.115	2	1.882	12	0	12	.002	15	-.128	3
443	13	max	118.944	3	5.443	9	87.247	1	0	1	.061	1	.172	2
444		min	-12.26	10	-30.344	2	1.882	12	0	12	.003	15	-.126	3
445	14	max	119.064	3	5.252	9	87.247	1	0	1	.08	1	.179	2
446		min	-12.127	10	-30.573	2	1.882	12	0	12	.004	15	-.123	3
447	15	max	119.184	3	5.062	9	87.247	1	0	1	.099	1	.186	2
448		min	-11.993	10	-30.801	2	1.882	12	0	12	.005	15	-.121	3
449	16	max	95.194	2	141.372	2	87.83	1	0	15	.119	1	.191	2
450		min	2.736	15	-206.069	3	1.883	12	0	1	.005	15	-.117	3
451	17	max	95.354	2	141.143	2	87.83	1	0	15	.138	1	.16	2
452		min	2.785	15	-206.241	3	1.883	12	0	1	.006	15	-.072	3
453	18	max	-7.319	15	370.911	2	92.609	1	0	2	.158	1	.081	2
454		min	-160.803	1	-165.678	3	2.269	12	0	3	.007	15	-.036	3
455	19	max	-7.271	15	370.682	2	92.609	1	0	2	.179	1	0	2
456		min	-160.643	1	-165.85	3	2.269	12	0	3	.008	15	0	3
457	M13	1	max	119.933	1	269.87	1	-7.28	15	0	.181	1	0	1
458		min	5.777	15	-342.669	3	-160.806	1	0	3	.008	15	0	3
459	2	max	119.933	1	190.299	1	-5.592	15	0	2	.059	1	.252	3
460		min	5.777	15	-241.572	3	-123.401	1	0	3	.003	15	-.198	1
461	3	max	119.933	1	110.728	1	-3.903	15	0	2	.002	3	.416	3
462		min	5.777	15	-140.475	3	-85.997	1	0	3	-.031	1	-.328	1
463	4	max	119.933	1	31.158	1	-2.215	15	0	2	-.002	12	.493	3
464		min	5.777	15	-39.377	3	-48.593	1	0	3	-.089	1	-.389	1
465	5	max	119.933	1	61.72	3	-.526	15	0	2	-.003	12	.484	3
466		min	5.777	15	-48.413	1	-11.188	1	0	3	-.115	1	-.381	1
467	6	max	119.933	1	162.817	3	26.216	1	0	2	-.004	12	.387	3
468		min	5.777	15	-127.983	1	.435	12	0	3	-.109	1	-.305	1
469	7	max	119.933	1	263.914	3	63.621	1	0	2	-.003	12	.203	3
470		min	5.777	15	-207.554	1	2.073	12	0	3	-.07	1	-.161	1
471	8	max	119.933	1	365.011	3	101.025	1	0	2	.002	2	.052	1
472		min	5.777	15	-287.125	1	3.711	12	0	3	0	3	-.067	3
473	9	max	119.933	1	466.109	3	138.429	1	0	2	.104	1	.334	1
474		min	5.777	15	-366.695	1	5.349	12	0	3	.004	12	-.425	3
475	10	max	119.933	1	567.206	3	175.834	1	0	2	.239	1	.684	1
476		min	5.777	15	-446.266	1	6.987	12	0	3	.009	12	-.87	3
477	11	max	92.462	1	366.695	1	-5.176	12	0	3	.1	1	.334	1
478		min	4.215	15	-466.109	3	-137.688	1	0	2	0	12	-.425	3
479	12	max	92.462	1	287.125	1	-3.538	12	0	3	.001	2	.052	1
480		min	4.215	15	-365.011	3	-100.284	1	0	2	-.004	3	-.067	3
481	13	max	92.462	1	207.554	1	-1.9	12	0	3	-.003	15	.203	3
482		min	4.215	15	-263.914	3	-62.88	1	0	2	-.073	1	-.161	1
483	14	max	92.462	1	127.983	1	-.262	12	0	3	-.005	15	.387	3
484		min	4.215	15	-162.817	3	-25.475	1	0	2	-.111	1	-.305	1
485	15	max	92.462	1	48.413	1	11.929	1	0	3	-.005	15	.484	3
486		min	4.215	15	-61.72	3	.562	15	0	2	-.117	1	-.381	1
487	16	max	92.462	1	39.377	3	49.334	1	0	3	-.004	12	.493	3
488		min	4.215	15	-31.158	1	2.251	15	0	2	-.09	1	-.389	1
489	17	max	92.462	1	140.475	3	86.738	1	0	3	0	12	.416	3
490		min	4.215	15	-110.729	1	3.939	15	0	2	-.032	1	-.328	1
491	18	max	92.462	1	241.572	3	124.142	1	0	3	.059	1	.252	3
492		min	4.215	15	-190.299	1	5.628	15	0	2	.003	15	-.198	1
493	19	max	92.462	1	342.669	3	161.547	1	0	3	.182	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16		min	4.215	15	-269.87	1	7.316	15	0	2	.008	15	0	3
495		1	max	-2.267	12	370.959	2	-7.271	15	0	3	.179	1	0	2
496			min	-92.214	1	-165.885	3	-160.661	1	0	2	.008	15	0	3
497		2	max	-2.267	12	261.598	2	-5.582	15	0	3	.056	1	.122	3
498			min	-92.214	1	-117.091	3	-123.257	1	0	2	.003	15	-.272	2
499		3	max	-2.267	12	152.236	2	-3.894	15	0	3	-.001	12	.202	3
500			min	-92.214	1	-68.297	3	-85.852	1	0	2	-.034	1	-.45	2
501		4	max	-2.267	12	42.875	2	-2.205	15	0	3	-.004	12	.239	3
502			min	-92.214	1	-19.503	3	-48.448	1	0	2	-.092	1	-.534	2
503		5	max	-2.267	12	29.291	3	-.517	15	0	3	-.005	15	.235	3
504			min	-92.214	1	-66.486	2	-11.043	1	0	2	-.117	1	-.524	2
505		6	max	-2.267	12	78.085	3	26.361	1	0	3	-.005	15	.189	3
506			min	-92.214	1	-175.847	2	.65	12	0	2	-.111	1	-.42	2
507		7	max	-2.267	12	126.879	3	63.765	1	0	3	-.003	15	.101	3
508			min	-92.214	1	-285.208	2	2.288	12	0	2	-.072	1	-.221	2
509		8	max	-2.267	12	175.673	3	101.17	1	0	3	.001	2	.071	2
510			min	-92.214	1	-394.569	2	3.926	12	0	2	-.003	3	-.03	3
511	9	max	-2.267	12	224.467	3	138.574	1	0	3	.102	1	.458	2	
512		min	-92.214	1	-503.93	2	5.564	12	0	2	.002	12	-.202	3	
513	10	max	-4.325	15	-13.527	15	175.979	1	0	15	.238	1	.939	2	
514		min	-94.71	1	-613.291	2	-11.121	3	0	2	.01	12	-.416	3	
515	11	max	-4.325	15	503.93	2	-5.792	12	0	2	.103	1	.458	2	
516		min	-94.71	1	-224.467	3	-138.234	1	0	3	.004	12	-.202	3	
517	12	max	-4.325	15	394.569	2	-4.153	12	0	2	.001	2	.071	2	
518		min	-94.71	1	-175.673	3	-100.83	1	0	3	0	9	-.03	3	
519	13	max	-4.325	15	285.208	2	-2.515	12	0	2	-.003	12	.101	3	
520		min	-94.71	1	-126.879	3	-63.426	1	0	3	-.071	1	-.221	2	
521	14	max	-4.325	15	175.847	2	-.877	12	0	2	-.005	12	.189	3	
522		min	-94.71	1	-78.085	3	-26.021	1	0	3	-.11	1	-.42	2	
523	15	max	-4.325	15	66.486	2	11.383	1	0	2	-.005	12	.235	3	
524		min	-94.71	1	-29.291	3	.53	15	0	3	-.116	1	-.524	2	
525	16	max	-4.325	15	19.503	3	48.788	1	0	2	-.003	12	.239	3	
526		min	-94.71	1	-42.875	2	2.219	15	0	3	-.09	1	-.534	2	
527	17	max	-4.325	15	68.297	3	86.192	1	0	2	0	12	.202	3	
528		min	-94.71	1	-152.236	2	3.907	15	0	3	-.032	1	-.45	2	
529	18	max	-4.325	15	117.091	3	123.596	1	0	2	.058	1	.122	3	
530		min	-94.71	1	-261.598	2	5.596	15	0	3	.003	15	-.272	2	
531	19	max	-4.325	15	165.885	3	161.001	1	0	2	.181	1	0	2	
532		min	-94.71	1	-370.959	2	7.284	15	0	3	.008	15	0	3	
533	M15	1	max	0	2	2.713	4	.036	3	0	1	0	1	0	1
534			min	-54.605	3	0	2	-.03	1	0	3	0	3	0	1
535		2	max	0	2	2.412	4	.036	3	0	1	0	1	0	2
536			min	-54.68	3	0	2	-.03	1	0	3	0	3	-.001	4
537		3	max	0	2	2.11	4	.036	3	0	1	0	1	0	2
538			min	-54.756	3	0	2	-.03	1	0	3	0	3	-.002	4
539		4	max	0	2	1.809	4	.036	3	0	1	0	1	0	2
540			min	-54.831	3	0	2	-.03	1	0	3	0	3	-.003	4
541		5	max	0	2	1.507	4	.036	3	0	1	0	1	0	2
542			min	-54.907	3	0	2	-.03	1	0	3	0	3	-.004	4
543		6	max	0	2	1.206	4	.036	3	0	1	0	1	0	2
544			min	-54.982	3	0	2	-.03	1	0	3	0	3	-.005	4
545		7	max	0	2	.904	4	.036	3	0	1	0	3	0	2
546			min	-55.058	3	0	2	-.03	1	0	3	0	1	-.005	4
547		8	max	0	2	.603	4	.036	3	0	1	0	3	0	2
548			min	-55.133	3	0	2	-.03	1	0	3	0	1	-.005	4
549		9	max	0	2	.301	4	.036	3	0	1	0	3	0	2
550		min	-55.209	3	0	2	-.03	1	0	3	0	1	-.006	4	





Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-54.001	3	-2.713	4	-.014	3	0	2	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.011	2	.017	1	-7.033e-5	15	NC	3	NC	3	
2			min	-.004	3	-.011	3	0	3	-1.541e-3	1	4010	2	2451.642	1	
3			2	max	.003	1	.01	2	.016	1	-6.719e-5	15	NC	3	NC	3
4				min	-.004	3	-.011	3	0	3	-1.473e-3	1	4387.587	2	2629.789	1
5			3	max	.003	1	.009	2	.015	1	-6.406e-5	15	NC	1	NC	3
6				min	-.003	3	-.01	3	0	3	-1.404e-3	1	4838.714	2	2840.938	1
7			4	max	.002	1	.008	2	.014	1	-6.092e-5	15	NC	1	NC	3
8				min	-.003	3	-.01	3	0	3	-1.336e-3	1	5381.435	2	3092.805	1
9			5	max	.002	1	.007	2	.013	1	-5.779e-5	15	NC	1	NC	3
10				min	-.003	3	-.009	3	0	3	-1.268e-3	1	6039.899	2	3395.693	1
11		6	max	.002	1	.006	2	.011	1	-5.465e-5	15	NC	1	NC	3	
12			min	-.003	3	-.009	3	0	3	-1.2e-3	1	6846.93	2	3763.597	1	
13		7	max	.002	1	.005	2	.01	1	-5.151e-5	15	NC	1	NC	3	
14			min	-.003	3	-.008	3	0	3	-1.131e-3	1	7847.996	2	4215.911	1	
15		8	max	.002	1	.005	2	.009	1	-4.838e-5	15	NC	1	NC	2	
16			min	-.002	3	-.008	3	0	3	-1.063e-3	1	9107.469	2	4780.142	1	
17		9	max	.002	1	.004	2	.008	1	-4.524e-5	15	NC	1	NC	2	
18			min	-.002	3	-.007	3	0	3	-9.948e-4	1	NC	1	5496.384	1	
19		10	max	.001	1	.003	2	.007	1	-4.211e-5	15	NC	1	NC	2	
20			min	-.002	3	-.007	3	0	3	-9.265e-4	1	NC	1	6425.023	1	
21		11	max	.001	1	.003	2	.006	1	-3.897e-5	15	NC	1	NC	2	
22			min	-.002	3	-.006	3	0	3	-8.582e-4	1	NC	1	7660.615	1	
23		12	max	.001	1	.002	2	.005	1	-3.583e-5	15	NC	1	NC	2	
24			min	-.001	3	-.005	3	0	3	-7.9e-4	1	NC	1	9358.492	1	
25		13	max	0	1	.002	2	.004	1	-3.27e-5	15	NC	1	NC	1	
26			min	-.001	3	-.005	3	0	3	-7.217e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.003	1	-2.956e-5	15	NC	1	NC	1	
28			min	-.001	3	-.004	3	0	3	-6.535e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-2.643e-5	15	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-5.852e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-2.329e-5	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-5.169e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-2.015e-5	15	NC	1	NC	1	
34			min	0	3	-.002	3	0	12	-4.487e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.702e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.804e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-1.091e-5	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.122e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.496e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	5.348e-6	12	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	1.819e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	7.923e-6	12	NC	1	NC	1
43			3	max	0	3	0	2	0	12	2.141e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	-.001	1	9.635e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	2.464e-4	1	NC	1	NC	1
46				min	0	2	-.003	3	-.001	1	1.113e-5	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	2.787e-4	1	NC	1	NC	1
48				min	0	2	-.004	3	-.001	1	1.262e-5	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	3.11e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.411e-5	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	3.433e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	1	1.56e-5	15	NC	1	NC	1
53		8	max	0	3	.001	2	0	3	3.756e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	1.709e-5	15	NC	1	NC	1
55		9	max	.001	3	.001	2	0	3	4.079e-4	1	NC	1	NC	1
56			min	0	2	-.007	3	0	2	1.858e-5	15	NC	1	NC	1
57		10	max	.001	3	.002	2	0	1	4.402e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	15	2.007e-5	15	NC	1	NC	1
59		11	max	.001	3	.002	2	.001	1	4.725e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	15	2.156e-5	15	NC	1	NC	1
61		12	max	.002	3	.003	2	.002	1	5.048e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	15	2.305e-5	15	NC	1	NC	1
63		13	max	.002	3	.004	2	.003	1	5.371e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	15	2.454e-5	15	NC	1	NC	1
65		14	max	.002	3	.004	2	.004	1	5.694e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	15	2.603e-5	15	NC	1	NC	1
67		15	max	.002	3	.005	2	.005	1	6.016e-4	1	NC	1	NC	2
68			min	-.002	2	-.009	3	0	15	2.752e-5	15	8562.915	2	8830.284	1
69		16	max	.002	3	.006	2	.006	1	6.339e-4	1	NC	1	NC	2
70			min	-.002	2	-.009	3	0	15	2.901e-5	15	7263.734	2	7367.217	1
71		17	max	.002	3	.007	2	.007	1	6.662e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	15	3.05e-5	15	6257.501	2	6309.596	1
73		18	max	.002	3	.008	2	.008	1	6.985e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	15	3.2e-5	15	5469.185	2	5518.985	1
75		19	max	.002	3	.01	2	.009	1	7.308e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	15	3.349e-5	15	4846.022	2	4913.097	1
77	M4	1	max	.002	1	.012	2	0	15	-5.113e-5	12	NC	1	NC	3
78			min	0	15	-.011	3	-.007	1	-1.167e-3	1	NC	1	2722.666	1
79		2	max	.002	1	.012	2	0	15	-5.113e-5	12	NC	1	NC	3
80			min	0	15	-.01	3	-.007	1	-1.167e-3	1	NC	1	2970.244	1
81		3	max	.002	1	.011	2	0	15	-5.113e-5	12	NC	1	NC	3
82			min	0	15	-.01	3	-.006	1	-1.167e-3	1	NC	1	3264.898	1
83		4	max	.002	1	.01	2	0	15	-5.113e-5	12	NC	1	NC	3
84			min	0	15	-.009	3	-.005	1	-1.167e-3	1	NC	1	3619.046	1
85		5	max	.002	1	.01	2	0	15	-5.113e-5	12	NC	1	NC	2
86			min	0	15	-.008	3	-.005	1	-1.167e-3	1	NC	1	4049.605	1
87		6	max	.001	1	.009	2	0	15	-5.113e-5	12	NC	1	NC	2
88			min	0	15	-.008	3	-.004	1	-1.167e-3	1	NC	1	4580.105	1
89		7	max	.001	1	.008	2	0	15	-5.113e-5	12	NC	1	NC	2
90			min	0	15	-.007	3	-.004	1	-1.167e-3	1	NC	1	5244.061	1
91		8	max	.001	1	.008	2	0	15	-5.113e-5	12	NC	1	NC	2
92			min	0	15	-.007	3	-.003	1	-1.167e-3	1	NC	1	6090.556	1
93		9	max	.001	1	.007	2	0	15	-5.113e-5	12	NC	1	NC	2
94			min	0	15	-.006	3	-.003	1	-1.167e-3	1	NC	1	7193.839	1
95		10	max	.001	1	.006	2	0	15	-5.113e-5	12	NC	1	NC	2
96			min	0	15	-.005	3	-.002	1	-1.167e-3	1	NC	1	8670.645	1
97		11	max	0	1	.006	2	0	15	-5.113e-5	12	NC	1	NC	1
98			min	0	15	-.005	3	-.002	1	-1.167e-3	1	NC	1	NC	1
99		12	max	0	1	.005	2	0	15	-5.113e-5	12	NC	1	NC	1
100			min	0	15	-.004	3	-.001	1	-1.167e-3	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	15	-5.113e-5	12	NC	1	NC	1
102			min	0	15	-.004	3	-.001	1	-1.167e-3	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	15	-5.113e-5	12	NC	1	NC	1
104			min	0	15	-.003	3	0	1	-1.167e-3	1	NC	1	NC	1
105		15	max	0	1	.003	2	0	15	-5.113e-5	12	NC	1	NC	1
106			min	0	15	-.002	3	0	1	-1.167e-3	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	15	-5.113e-5	12	NC	1	NC	1
108			min	0	15	-.002	3	0	1	-1.167e-3	1	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-5.113e-5	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-1.167e-3	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-5.113e-5	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-1.167e-3	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-5.113e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.167e-3	1	NC	1	NC	1
115	M6	1	max	.009	1	.039	2	.005	1	3.493e-4	3	NC	3	NC	2
116			min	-.013	3	-.036	3	-.004	3	1.512e-6	10	1099.747	2	8079.769	1
117		2	max	.009	1	.036	2	.005	1	3.374e-4	3	NC	3	NC	2
118			min	-.012	3	-.034	3	-.004	3	6.146e-7	10	1176.997	2	8784.209	1
119		3	max	.008	1	.034	2	.004	1	3.254e-4	3	NC	3	NC	2
120			min	-.011	3	-.032	3	-.004	3	-2.83e-7	10	1265.535	2	9617.218	1
121		4	max	.008	1	.031	2	.004	1	3.135e-4	3	NC	3	NC	1
122			min	-.01	3	-.03	3	-.003	3	-3.24e-6	2	1367.606	2	NC	1
123		5	max	.007	1	.029	2	.004	1	3.016e-4	3	NC	3	NC	1
124			min	-.01	3	-.028	3	-.003	3	-6.585e-6	2	1486.104	2	NC	1
125		6	max	.007	1	.026	2	.003	1	2.897e-4	3	NC	3	NC	1
126			min	-.009	3	-.026	3	-.003	3	-9.931e-6	2	1624.814	2	NC	1
127		7	max	.006	1	.024	2	.003	1	2.777e-4	3	NC	3	NC	1
128			min	-.008	3	-.024	3	-.003	3	-1.328e-5	2	1788.782	2	NC	1
129		8	max	.006	1	.021	2	.002	1	2.658e-4	3	NC	3	NC	1
130			min	-.008	3	-.022	3	-.002	3	-1.662e-5	2	1984.889	2	NC	1
131		9	max	.005	1	.019	2	.002	1	2.539e-4	3	NC	3	NC	1
132			min	-.007	3	-.02	3	-.002	3	-1.997e-5	2	2222.765	2	NC	1
133		10	max	.005	1	.017	2	.002	1	2.419e-4	3	NC	3	NC	1
134			min	-.006	3	-.018	3	-.002	3	-2.331e-5	2	2516.308	2	NC	1
135		11	max	.004	1	.015	2	.001	1	2.3e-4	3	NC	3	NC	1
136			min	-.006	3	-.016	3	-.002	3	-2.666e-5	2	2886.355	2	NC	1
137		12	max	.004	1	.013	2	.001	1	2.181e-4	3	NC	3	NC	1
138			min	-.005	3	-.014	3	-.001	3	-3.e-5	2	3365.627	2	NC	1
139		13	max	.003	1	.011	2	0	1	2.061e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.001	3	-3.335e-5	2	4008.626	2	NC	1
141		14	max	.003	1	.009	2	0	1	1.942e-4	3	NC	3	NC	1
142			min	-.003	3	-.01	3	0	3	-3.67e-5	2	4913.41	2	NC	1
143		15	max	.002	1	.007	2	0	1	1.823e-4	3	NC	3	NC	1
144			min	-.003	3	-.008	3	0	3	-4.004e-5	2	6276.019	2	NC	1
145		16	max	.002	1	.005	2	0	1	1.703e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-4.339e-5	2	8553.766	2	NC	1
147		17	max	.001	1	.003	2	0	1	1.584e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-4.673e-5	2	NC	1	NC	1
149		18	max	0	1	.002	2	0	1	1.465e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-5.008e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.345e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-5.342e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.536e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-6.409e-5	3	NC	1	NC	1
155		2	max	0	3	.002	2	0	3	2.114e-5	2	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-4.643e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.034e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	2	-2.878e-5	3	NC	1	NC	1
159		4	max	.001	3	.005	2	0	3	2.038e-5	1	NC	1	NC	1
160			min	-.001	2	-.007	3	0	2	-1.113e-5	3	9612.452	2	NC	1
161		5	max	.002	3	.006	2	.001	3	2.041e-5	1	NC	1	NC	1
162			min	-.002	2	-.009	3	0	2	7.772e-7	15	7252.888	2	NC	1
163		6	max	.002	3	.008	2	.001	3	2.418e-5	3	NC	3	NC	1
164			min	-.002	2	-.011	3	0	1	9.132e-7	15	5809.967	2	NC	1
165		7	max	.003	3	.01	2	.001	3	4.183e-5	3	NC	3	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.013	3	0	1	7.904e-8	2	4827.446	2	NC	1
167		8	max	.003	3	.011	2	.002	3	5.948e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-4.134e-6	2	4110.229	2	NC	1
169		9	max	.003	3	.013	2	.002	3	7.714e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-8.347e-6	2	3561.002	2	NC	1
171		10	max	.004	3	.015	2	.002	3	9.479e-5	3	NC	3	NC	1
172			min	-.004	2	-.018	3	-.001	1	-1.256e-5	2	3125.786	2	NC	1
173		11	max	.004	3	.017	2	.002	3	1.124e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.677e-5	2	2772.176	2	NC	1
175		12	max	.005	3	.019	2	.002	3	1.301e-4	3	NC	3	NC	1
176			min	-.005	2	-.021	3	-.001	1	-2.099e-5	2	2479.472	2	NC	1
177		13	max	.005	3	.021	2	.002	3	1.477e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-2.52e-5	2	2233.76	2	NC	1
179		14	max	.006	3	.023	2	.002	3	1.654e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-2.941e-5	2	2025.282	2	NC	1
181		15	max	.006	3	.025	2	.002	3	1.831e-4	3	NC	3	NC	1
182			min	-.007	2	-.025	3	-.002	1	-3.363e-5	2	1846.946	2	NC	1
183		16	max	.007	3	.027	2	.002	3	2.007e-4	3	NC	3	NC	1
184			min	-.007	2	-.026	3	-.002	1	-3.784e-5	2	1693.443	2	NC	1
185		17	max	.007	3	.03	2	.002	3	2.184e-4	3	NC	3	NC	1
186			min	-.008	2	-.027	3	-.002	1	-4.205e-5	2	1560.698	2	NC	1
187		18	max	.007	3	.032	2	.002	3	2.36e-4	3	NC	3	NC	1
188			min	-.008	2	-.028	3	-.002	1	-4.627e-5	2	1445.519	2	NC	1
189		19	max	.008	3	.034	2	.002	3	2.537e-4	3	NC	3	NC	1
190			min	-.009	2	-.029	3	-.003	1	-5.048e-5	2	1345.369	2	NC	1
191	M8	1	max	.005	1	.045	2	.003	1	-7.936e-6	10	NC	1	NC	2
192			min	0	15	-.035	3	-.001	3	-2.128e-4	1	NC	1	7238.476	1
193		2	max	.005	1	.042	2	.002	1	-7.936e-6	10	NC	1	NC	2
194			min	0	15	-.033	3	-.001	3	-2.128e-4	1	NC	1	7891.824	1
195		3	max	.004	1	.04	2	.002	1	-7.936e-6	10	NC	1	NC	2
196			min	0	15	-.031	3	-.001	3	-2.128e-4	1	NC	1	8669.639	1
197		4	max	.004	1	.037	2	.002	1	-7.936e-6	10	NC	1	NC	2
198			min	0	15	-.029	3	-.001	3	-2.128e-4	1	NC	1	9604.708	1
199		5	max	.004	1	.035	2	.002	1	-7.936e-6	10	NC	1	NC	1
200			min	0	15	-.027	3	0	3	-2.128e-4	1	NC	1	NC	1
201		6	max	.004	1	.032	2	.002	1	-7.936e-6	10	NC	1	NC	1
202			min	0	15	-.025	3	0	3	-2.128e-4	1	NC	1	NC	1
203		7	max	.003	1	.03	2	.001	1	-7.936e-6	10	NC	1	NC	1
204			min	0	15	-.023	3	0	3	-2.128e-4	1	NC	1	NC	1
205		8	max	.003	1	.027	2	.001	1	-7.936e-6	10	NC	1	NC	1
206			min	0	15	-.021	3	0	3	-2.128e-4	1	NC	1	NC	1
207		9	max	.003	1	.025	2	.001	1	-7.936e-6	10	NC	1	NC	1
208			min	0	15	-.02	3	0	3	-2.128e-4	1	NC	1	NC	1
209		10	max	.002	1	.022	2	0	1	-7.936e-6	10	NC	1	NC	1
210			min	0	15	-.018	3	0	3	-2.128e-4	1	NC	1	NC	1
211		11	max	.002	1	.02	2	0	1	-7.936e-6	10	NC	1	NC	1
212			min	0	15	-.016	3	0	3	-2.128e-4	1	NC	1	NC	1
213		12	max	.002	1	.017	2	0	1	-7.936e-6	10	NC	1	NC	1
214			min	0	15	-.014	3	0	3	-2.128e-4	1	NC	1	NC	1
215		13	max	.002	1	.015	2	0	1	-7.936e-6	10	NC	1	NC	1
216			min	0	15	-.012	3	0	3	-2.128e-4	1	NC	1	NC	1
217		14	max	.001	1	.012	2	0	1	-7.936e-6	10	NC	1	NC	1
218			min	0	15	-.01	3	0	3	-2.128e-4	1	NC	1	NC	1
219		15	max	.001	1	.01	2	0	1	-7.936e-6	10	NC	1	NC	1
220			min	0	15	-.008	3	0	3	-2.128e-4	1	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-7.936e-6	10	NC	1	NC	1
222			min	0	15	-.006	3	0	3	-2.128e-4	1	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.005	2	0	1	-7.936e-6	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.128e-4	1	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-7.936e-6	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.128e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-7.936e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.128e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.011	2	0	3	1.261e-3	1	NC	3	NC	1
230			min	-.004	3	-.011	3	-.002	1	-3.21e-4	3	4011.422	2	NC	1
231		2	max	.003	1	.01	2	0	3	1.195e-3	1	NC	3	NC	1
232			min	-.003	3	-.011	3	-.002	1	-3.105e-4	3	4389.222	2	NC	1
233		3	max	.003	1	.009	2	0	3	1.129e-3	1	NC	3	NC	1
234			min	-.003	3	-.01	3	-.002	1	-2.999e-4	3	4840.62	2	NC	1
235		4	max	.003	1	.008	2	0	3	1.063e-3	1	NC	1	NC	1
236			min	-.003	3	-.01	3	-.002	1	-2.893e-4	3	5383.69	2	NC	1
237		5	max	.002	1	.007	2	0	3	9.968e-4	1	NC	1	NC	1
238			min	-.003	3	-.009	3	-.002	1	-2.788e-4	3	6042.606	2	NC	1
239		6	max	.002	1	.006	2	0	3	9.308e-4	1	NC	1	NC	1
240			min	-.003	3	-.009	3	-.002	1	-2.682e-4	3	6850.234	2	NC	1
241		7	max	.002	1	.005	2	0	3	8.648e-4	1	NC	1	NC	1
242			min	-.002	3	-.008	3	-.002	1	-2.576e-4	3	7852.097	2	NC	1
243		8	max	.002	1	.005	2	0	3	7.989e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	-.002	1	-2.471e-4	3	9112.658	2	NC	1
245		9	max	.002	1	.004	2	0	3	7.329e-4	1	NC	1	NC	1
246			min	-.002	3	-.007	3	-.002	1	-2.365e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	6.669e-4	1	NC	1	NC	1
248			min	-.002	3	-.007	3	-.001	1	-2.259e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	6.01e-4	1	NC	1	NC	1
250			min	-.002	3	-.006	3	-.001	1	-2.154e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	5.35e-4	1	NC	1	NC	1
252			min	-.001	3	-.006	3	0	1	-2.048e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	4.691e-4	1	NC	1	NC	1
254			min	-.001	3	-.005	3	0	1	-1.942e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	4.031e-4	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-1.837e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	3.371e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-1.731e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.712e-4	1	NC	1	NC	1
260			min	0	3	-.003	3	0	1	-1.625e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	2.052e-4	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-1.52e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.392e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.414e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	7.326e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.308e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	6.255e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.638e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	11	4.288e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-9.885e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	2.322e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.613e-4	1	NC	1	NC	1
273		4	max	0	3	0	2	0	10	3.551e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-2.238e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	-1.092e-5	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-2.863e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	15	-1.584e-5	15	NC	1	NC	1
278			min	0	2	-.005	3	-.002	1	-3.487e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	15	-1.88e-5	15	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	1	-4.112e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	15	-2.176e-5	15	NC	1	NC	1
282			min	0	2	-.006	3	-.003	1	-4.737e-4	1	NC	1	NC	1
283		9	max	.001	3	.001	2	0	15	-2.472e-5	15	NC	1	NC	1
284			min	0	2	-.007	3	-.004	1	-5.362e-4	1	NC	1	NC	1
285		10	max	.001	3	.002	2	0	15	-2.768e-5	15	NC	1	NC	2
286			min	-.001	2	-.007	3	-.006	1	-5.986e-4	1	NC	1	8203.085	1
287		11	max	.001	3	.002	2	0	15	-3.064e-5	15	NC	1	NC	2
288			min	-.001	2	-.008	3	-.007	1	-6.611e-4	1	NC	1	6714.762	1
289		12	max	.002	3	.003	2	0	15	-3.36e-5	15	NC	1	NC	2
290			min	-.001	2	-.008	3	-.008	1	-7.236e-4	1	NC	1	5637.325	1
291		13	max	.002	3	.004	2	0	15	-3.656e-5	15	NC	1	NC	2
292			min	-.001	2	-.008	3	-.01	1	-7.861e-4	1	NC	1	4831.982	1
293		14	max	.002	3	.004	2	0	15	-3.951e-5	15	NC	1	NC	2
294			min	-.002	2	-.009	3	-.011	1	-8.485e-4	1	NC	1	4214.427	1
295		15	max	.002	3	.005	2	0	15	-4.247e-5	15	NC	1	NC	3
296			min	-.002	2	-.009	3	-.012	1	-9.11e-4	1	8573.683	2	3730.996	1
297		16	max	.002	3	.006	2	0	15	-4.543e-5	15	NC	1	NC	3
298			min	-.002	2	-.009	3	-.014	1	-9.735e-4	1	7272.13	2	3346.189	1
299		17	max	.002	3	.007	2	0	15	-4.839e-5	15	NC	1	NC	3
300			min	-.002	2	-.009	3	-.015	1	-1.036e-3	1	6264.216	2	3035.767	1
301		18	max	.002	3	.008	2	0	15	-5.135e-5	15	NC	1	NC	3
302			min	-.002	2	-.009	3	-.017	1	-1.098e-3	1	5474.687	2	2782.728	1
303		19	max	.002	3	.009	2	0	15	-5.431e-5	15	NC	3	NC	3
304			min	-.002	2	-.009	3	-.018	1	-1.161e-3	1	4850.633	2	2574.888	1
305	M12	1	max	.002	1	.012	2	.015	1	1.148e-3	1	NC	1	NC	3
306			min	0	15	-.011	3	0	15	5.468e-5	15	NC	1	1313.289	1
307		2	max	.002	1	.012	2	.013	1	1.148e-3	1	NC	1	NC	3
308			min	0	15	-.01	3	0	15	5.468e-5	15	NC	1	1432.271	1
309		3	max	.002	1	.011	2	.012	1	1.148e-3	1	NC	1	NC	3
310			min	0	15	-.01	3	0	15	5.468e-5	15	NC	1	1573.897	1
311		4	max	.002	1	.01	2	.011	1	1.148e-3	1	NC	1	NC	3
312			min	0	15	-.009	3	0	15	5.468e-5	15	NC	1	1744.137	1
313		5	max	.002	1	.01	2	.01	1	1.148e-3	1	NC	1	NC	3
314			min	0	15	-.009	3	0	15	5.468e-5	15	NC	1	1951.124	1
315		6	max	.001	1	.009	2	.009	1	1.148e-3	1	NC	1	NC	3
316			min	0	15	-.008	3	0	15	5.468e-5	15	NC	1	2206.167	1
317		7	max	.001	1	.008	2	.008	1	1.148e-3	1	NC	1	NC	3
318			min	0	15	-.007	3	0	15	5.468e-5	15	NC	1	2525.377	1
319		8	max	.001	1	.008	2	.007	1	1.148e-3	1	NC	1	NC	3
320			min	0	15	-.007	3	0	15	5.468e-5	15	NC	1	2932.348	1
321		9	max	.001	1	.007	2	.006	1	1.148e-3	1	NC	1	NC	3
322			min	0	15	-.006	3	0	15	5.468e-5	15	NC	1	3462.767	1
323		10	max	.001	1	.006	2	.005	1	1.148e-3	1	NC	1	NC	2
324			min	0	15	-.005	3	0	15	5.468e-5	15	NC	1	4172.743	1
325		11	max	0	1	.006	2	.004	1	1.148e-3	1	NC	1	NC	2
326			min	0	15	-.005	3	0	15	5.468e-5	15	NC	1	5154.694	1
327		12	max	0	1	.005	2	.003	1	1.148e-3	1	NC	1	NC	2
328			min	0	15	-.004	3	0	15	5.468e-5	15	NC	1	6569.849	1
329		13	max	0	1	.004	2	.002	1	1.148e-3	1	NC	1	NC	2
330			min	0	15	-.004	3	0	15	5.468e-5	15	NC	1	8720.67	1
331		14	max	0	1	.003	2	.002	1	1.148e-3	1	NC	1	NC	1
332			min	0	15	-.003	3	0	15	5.468e-5	15	NC	1	NC	1
333		15	max	0	1	.003	2	.001	1	1.148e-3	1	NC	1	NC	1
334			min	0	15	-.002	3	0	15	5.468e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	1.148e-3	1	NC	1	NC	1
336			min	0	15	-.002	3	0	15	5.468e-5	15	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	1.148e-3	1	NC	1	NC	1
338			min	0	15	-.001	3	0	15	5.468e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.148e-3	1	NC	1	NC	1
340			min	0	15	0	3	0	15	5.468e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.148e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	5.468e-5	15	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.002	3	2.208e-2	1	NC	1	NC	1
344			min	-.009	2	-.024	2	-.006	1	-2.791e-2	3	NC	1	NC	1
345		2	max	.01	3	.016	3	.001	3	1.048e-2	1	NC	4	NC	2
346			min	-.009	2	-.015	2	-.013	1	-1.383e-2	3	4780.778	2	6584.853	1
347		3	max	.01	3	.007	3	0	3	-1.726e-5	12	NC	4	NC	2
348			min	-.009	2	-.005	2	-.017	1	-9.079e-4	1	2454.309	2	3994.039	1
349		4	max	.009	3	.003	1	0	3	-1.213e-5	12	NC	4	NC	3
350			min	-.009	2	-.002	3	-.02	1	-7.843e-4	1	1717.451	2	3306.194	1
351		5	max	.009	3	.01	2	0	3	-7.003e-6	12	NC	5	NC	3
352			min	-.009	2	-.008	3	-.02	1	-6.608e-4	1	1361.525	2	3176.267	1
353		6	max	.009	3	.016	2	0	3	-7.543e-7	3	NC	5	NC	3
354			min	-.009	2	-.014	3	-.019	1	-5.372e-4	1	1158.958	2	3401.028	1
355		7	max	.009	3	.021	2	0	3	6.904e-6	3	NC	5	NC	2
356			min	-.009	2	-.018	3	-.017	1	-4.137e-4	1	1034.992	2	4054.055	1
357		8	max	.009	3	.025	2	0	3	1.456e-5	3	NC	5	NC	2
358			min	-.009	2	-.021	3	-.014	1	-2.902e-4	1	958.511	2	5576.636	1
359		9	max	.009	3	.027	2	0	3	2.222e-5	3	NC	5	NC	1
360			min	-.009	2	-.023	3	-.01	1	-1.666e-4	1	915.006	2	NC	1
361		10	max	.009	3	.028	2	0	3	2.988e-5	3	NC	5	NC	1
362			min	-.009	2	-.024	3	-.005	1	-4.308e-5	1	897.862	2	NC	1
363		11	max	.009	3	.027	2	0	3	8.046e-5	1	NC	5	NC	1
364			min	-.009	2	-.023	3	-.001	1	4.087e-6	15	905.278	2	NC	1
365		12	max	.009	3	.026	2	.003	1	2.04e-4	1	NC	5	NC	2
366			min	-.009	2	-.021	3	0	15	9.692e-6	15	939.628	2	6309.092	1
367		13	max	.009	3	.022	2	.006	1	3.275e-4	1	NC	5	NC	2
368			min	-.009	2	-.018	3	0	15	1.53e-5	15	1008.74	2	4401.202	1
369		14	max	.009	3	.017	2	.008	1	4.511e-4	1	NC	5	NC	3
370			min	-.009	2	-.014	3	0	15	2.09e-5	15	1130.452	2	3616.9	1
371		15	max	.009	3	.01	2	.01	1	5.746e-4	1	NC	4	NC	3
372			min	-.009	2	-.008	3	0	15	2.651e-5	15	1346.341	2	3336.246	1
373		16	max	.009	3	.002	1	.009	1	6.584e-4	1	NC	4	NC	3
374			min	-.009	2	-.002	3	0	15	3.034e-5	15	1756.301	1	3442.383	1
375		17	max	.009	3	.006	3	.007	1	2.349e-5	3	NC	4	NC	2
376			min	-.009	2	-.008	2	0	15	-2.03e-4	1	2469.426	1	4133.839	1
377		18	max	.009	3	.014	3	.003	1	1.5e-2	2	NC	2	NC	2
378			min	-.009	2	-.02	2	0	15	-6.835e-3	3	4772.44	1	6787.143	1
379		19	max	.009	3	.023	3	0	3	3.039e-2	2	NC	1	NC	1
380			min	-.009	2	-.032	2	-.004	1	-1.382e-2	3	5850.789	2	NC	1
381	M5	1	max	.031	3	.088	3	.002	3	1.602e-6	3	NC	1	NC	1
382			min	-.035	2	-.083	2	-.007	1	5.472e-8	10	3474.446	3	NC	1
383		2	max	.031	3	.054	3	.003	3	9.62e-5	3	NC	5	NC	1
384			min	-.035	2	-.05	2	-.006	1	-5.222e-5	1	1418.769	2	NC	1
385		3	max	.03	3	.022	3	.004	3	1.889e-4	3	NC	5	NC	1
386			min	-.035	2	-.018	2	-.005	1	-1.045e-4	1	727.894	2	NC	1
387		4	max	.03	3	.01	2	.005	3	1.818e-4	3	NC	5	NC	1
388			min	-.035	2	-.005	3	-.005	1	-9.986e-5	1	508.784	2	NC	1
389		5	max	.03	3	.034	2	.005	3	1.747e-4	3	NC	5	NC	1
390			min	-.035	2	-.028	3	-.004	1	-9.526e-5	1	402.897	2	NC	1
391		6	max	.03	3	.054	2	.006	3	1.676e-4	3	NC	15	NC	1
392			min	-.035	2	-.046	3	-.004	1	-9.066e-5	1	342.599	2	NC	1
393		7	max	.03	3	.071	2	.006	3	1.605e-4	3	NC	15	NC	1



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Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394		min	-.035	2	-.06	3	-.003	1	-8.606e-5	1	305.663	2	NC	1
395	8	max	.03	3	.083	2	.006	3	1.534e-4	3	NC	15	NC	1
396		min	-.035	2	-.07	3	-.003	1	-8.146e-5	1	282.836	2	NC	1
397	9	max	.03	3	.091	2	.005	3	1.462e-4	3	NC	15	NC	1
398		min	-.035	2	-.075	3	-.003	1	-7.686e-5	1	269.8	2	NC	1
399	10	max	.03	3	.095	2	.005	3	1.391e-4	3	NC	15	NC	1
400		min	-.035	2	-.077	3	-.002	1	-7.226e-5	1	264.584	2	NC	1
401	11	max	.03	3	.093	2	.005	3	1.32e-4	3	NC	15	NC	1
402		min	-.035	2	-.074	3	-.002	1	-6.766e-5	1	266.648	2	NC	1
403	12	max	.03	3	.087	2	.004	3	1.249e-4	3	NC	15	NC	1
404		min	-.035	2	-.068	3	-.002	1	-6.306e-5	1	276.693	2	NC	1
405	13	max	.029	3	.075	2	.004	3	1.178e-4	3	NC	15	NC	1
406		min	-.034	2	-.058	3	-.002	1	-5.845e-5	1	297.045	2	NC	1
407	14	max	.029	3	.058	2	.003	3	1.107e-4	3	NC	15	NC	1
408		min	-.034	2	-.044	3	-.002	1	-5.385e-5	1	333.021	2	NC	1
409	15	max	.029	3	.035	2	.003	3	1.036e-4	3	NC	5	NC	1
410		min	-.034	2	-.027	3	-.002	1	-4.925e-5	1	397.068	2	NC	1
411	16	max	.029	3	.007	1	.002	3	9.19e-5	3	NC	5	NC	1
412		min	-.034	2	-.006	3	-.003	1	-5.418e-5	1	523.931	2	NC	1
413	17	max	.029	3	.019	3	.001	3	-1.209e-5	15	NC	5	NC	1
414		min	-.035	2	-.028	2	-.003	1	-2.859e-4	1	848.461	1	NC	1
415	18	max	.029	3	.046	3	0	3	-6.218e-6	15	NC	5	NC	1
416		min	-.034	2	-.068	2	-.003	1	-1.465e-4	1	1669.064	3	NC	1
417	19	max	.029	3	.074	3	0	3	-3.953e-8	15	NC	3	NC	1
418		min	-.034	2	-.111	2	-.003	1	-3.37e-7	3	1669.669	2	NC	1
419	M9	1	max	.01	.027	3	.002	3	2.791e-2	3	NC	1	NC	1
420		min	-.009	2	-.024	2	-.008	1	-2.208e-2	1	NC	1	NC	1
421	2	max	.01	3	.016	3	0	3	1.38e-2	3	NC	4	NC	2
422		min	-.009	2	-.015	2	-.002	1	-1.076e-2	1	4781.965	2	7630.805	1
423	3	max	.01	3	.006	3	.003	1	3.566e-4	1	NC	4	NC	2
424		min	-.009	2	-.005	2	0	3	-4.872e-5	3	2454.934	2	4739.502	1
425	4	max	.01	3	.003	1	.005	1	2.522e-4	1	NC	4	NC	3
426		min	-.009	2	-.002	3	0	3	-5.495e-5	3	1717.886	2	4018.775	1
427	5	max	.009	3	.01	2	.005	1	1.478e-4	1	NC	5	NC	3
428		min	-.009	2	-.009	3	-.002	3	-6.119e-5	3	1361.852	2	3987.548	1
429	6	max	.009	3	.016	2	.004	1	4.346e-5	1	NC	5	NC	3
430		min	-.009	2	-.014	3	-.002	3	-6.743e-5	3	1159.214	2	4483.57	1
431	7	max	.009	3	.021	2	.002	1	4.602e-6	10	NC	5	NC	2
432		min	-.009	2	-.018	3	-.003	3	-7.367e-5	3	1035.196	2	5818.243	1
433	8	max	.009	3	.025	2	0	2	-7.477e-6	10	NC	5	NC	2
434		min	-.009	2	-.021	3	-.003	3	-1.653e-4	1	958.672	2	9660.225	1
435	9	max	.009	3	.027	2	0	10	-1.241e-5	15	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	1	-2.697e-4	1	915.13	2	NC	1
437	10	max	.009	3	.028	2	0	15	-1.714e-5	15	NC	5	NC	1
438		min	-.009	2	-.024	3	-.008	1	-3.741e-4	1	897.948	2	9636.138	3
439	11	max	.009	3	.027	2	0	15	-2.187e-5	15	NC	5	NC	2
440		min	-.009	2	-.023	3	-.011	1	-4.784e-4	1	905.321	2	7020.478	1
441	12	max	.009	3	.026	2	0	15	-2.661e-5	15	NC	5	NC	2
442		min	-.009	2	-.021	3	-.015	1	-5.828e-4	1	939.615	2	4601.91	1
443	13	max	.009	3	.022	2	0	15	-3.134e-5	15	NC	5	NC	2
444		min	-.009	2	-.018	3	-.017	1	-6.872e-4	1	1008.645	2	3594.259	1
445	14	max	.009	3	.017	2	0	15	-3.607e-5	15	NC	5	NC	3
446		min	-.009	2	-.014	3	-.019	1	-7.916e-4	1	1130.216	2	3135.042	1
447	15	max	.009	3	.01	2	0	15	-4.08e-5	15	NC	4	NC	3
448		min	-.009	2	-.008	3	-.019	1	-8.96e-4	1	1345.82	2	2999.984	1
449	16	max	.009	3	.002	1	0	15	-4.402e-5	15	NC	4	NC	3
450		min	-.009	2	-.002	3	-.018	1	-9.685e-4	1	1756.169	1	3173.028	1



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Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.009	3	.006	3	0	15	6.344e-5	3	NC	4	NC	2
452			min	-.009	2	-.008	2	-.015	1	-2.84e-4	1	2469.261	1	3878.542	1
453		18	max	.009	3	.014	3	0	15	6.88e-3	3	NC	2	NC	2
454			min	-.009	2	-.02	2	-.01	1	-1.512e-2	2	4772.132	1	6452.25	1
455		19	max	.009	3	.023	3	0	3	1.382e-2	3	NC	1	NC	1
456			min	-.009	2	-.032	2	-.002	1	-3.039e-2	2	5877.551	2	NC	1
457	M13	1	max	.008	1	.027	3	.01	3	3.942e-3	3	NC	1	NC	1
458			min	-.002	3	-.024	2	-.009	2	-3.722e-3	2	NC	1	NC	1
459		2	max	.008	1	.305	3	.057	1	4.942e-3	3	NC	5	NC	3
460			min	-.002	3	-.242	1	.001	10	-4.702e-3	2	667.951	3	3023.002	1
461		3	max	.008	1	.532	3	.146	1	5.942e-3	3	NC	15	NC	3
462			min	-.002	3	-.422	1	.007	15	-5.683e-3	2	367.862	3	1239.809	1
463		4	max	.008	1	.674	3	.222	1	6.942e-3	3	NC	15	NC	3
464			min	-.002	3	-.535	1	.01	15	-6.663e-3	2	287.511	3	823.347	1
465		5	max	.007	1	.713	3	.26	1	7.942e-3	3	NC	15	NC	3
466			min	-.002	3	-.566	1	.012	15	-7.644e-3	2	271.089	3	705.233	1
467		6	max	.007	1	.652	3	.248	1	8.941e-3	3	NC	15	NC	5
468			min	-.002	3	-.519	1	.012	15	-8.624e-3	2	297.498	3	737.42	1
469		7	max	.007	1	.511	3	.19	1	9.941e-3	3	NC	5	NC	5
470			min	-.002	3	-.409	1	.009	15	-9.604e-3	2	383.889	3	957.72	1
471		8	max	.007	1	.329	3	.102	1	1.094e-2	3	NC	5	NC	5
472			min	-.002	3	-.267	1	-.003	10	-1.058e-2	2	614.642	3	1743.79	1
473		9	max	.007	1	.163	3	.029	3	1.194e-2	3	NC	5	NC	2
474			min	-.002	3	-.138	2	-.017	2	-1.157e-2	2	1363.716	3	8981.911	1
475		10	max	.007	1	.088	3	.031	3	1.294e-2	3	NC	4	NC	1
476			min	-.002	3	-.083	2	-.035	2	-1.255e-2	2	3048.853	3	7376.22	2
477		11	max	.007	1	.163	3	.036	3	1.194e-2	3	NC	5	NC	2
478			min	-.002	3	-.138	2	-.017	2	-1.157e-2	2	1363.714	3	7129.554	3
479		12	max	.007	1	.329	3	.11	1	1.094e-2	3	NC	5	NC	5
480			min	-.002	3	-.267	1	-.003	10	-1.059e-2	2	614.641	3	1623.512	1
481		13	max	.006	1	.511	3	.199	1	9.944e-3	3	NC	5	NC	5
482			min	-.002	3	-.409	1	.01	10	-9.605e-3	2	383.889	3	913.866	1
483		14	max	.006	1	.652	3	.257	1	8.945e-3	3	NC	15	NC	15
484			min	-.002	3	-.519	1	.012	15	-8.624e-3	2	297.498	3	710.867	1
485		15	max	.006	1	.713	3	.268	1	7.946e-3	3	NC	15	NC	15
486			min	-.002	3	-.566	1	.013	15	-7.644e-3	2	271.089	3	683.244	1
487		16	max	.006	1	.674	3	.228	1	6.947e-3	3	NC	15	NC	5
488			min	-.002	3	-.534	1	.011	15	-6.664e-3	2	287.511	3	799.291	1
489		17	max	.006	1	.532	3	.15	1	5.948e-3	3	NC	15	NC	3
490			min	-.002	3	-.422	1	.007	15	-5.683e-3	2	367.862	3	1202.657	1
491		18	max	.006	1	.305	3	.06	1	4.949e-3	3	NC	5	NC	3
492			min	-.002	3	-.242	1	.001	10	-4.703e-3	2	667.95	3	2915.221	1
493		19	max	.006	1	.027	3	.01	3	3.95e-3	3	NC	1	NC	1
494			min	-.002	3	-.024	2	-.009	2	-3.723e-3	2	NC	1	NC	1
495	M16	1	max	.002	1	.023	3	.009	3	4.74e-3	2	NC	1	NC	1
496			min	0	3	-.032	2	-.009	2	-3.297e-3	3	NC	1	NC	1
497		2	max	.002	1	.161	3	.061	1	6.005e-3	2	NC	5	NC	3
498			min	0	3	-.336	2	.001	10	-4.119e-3	3	613.328	2	2847.492	1
499		3	max	.002	1	.274	3	.152	1	7.27e-3	2	NC	15	NC	3
500			min	0	3	-.583	2	.007	15	-4.94e-3	3	337.581	2	1188.278	1
501		4	max	.002	1	.346	3	.23	1	8.534e-3	2	NC	15	NC	5
502			min	0	3	-.738	2	.011	15	-5.762e-3	3	263.559	2	794.523	1
503		5	max	.002	1	.368	3	.268	1	9.799e-3	2	NC	15	NC	15
504			min	0	3	-.782	2	.013	15	-6.584e-3	3	248.049	2	682.173	1
505		6	max	.003	1	.341	3	.256	1	1.106e-2	2	NC	15	NC	15
506			min	0	3	-.718	2	.012	15	-7.406e-3	3	271.336	2	712.777	1
507		7	max	.003	1	.275	3	.197	1	1.233e-2	2	NC	5	NC	5



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-567	2	.01	15	-8.227e-3	3	347.938	2	921.351	1
509	8	max	.003	1	.189	3	.108	1	1.359e-2	2	NC	5	NC	5
510		min	0	3	-.371	2	-.003	10	-9.049e-3	3	549.017	2	1653.762	1
511	9	max	.003	1	.11	3	.033	3	1.486e-2	2	NC	5	NC	2
512		min	0	3	-.192	2	-.017	2	-9.871e-3	3	1165.414	2	7754.225	3
513	10	max	.003	1	.074	3	.029	3	1.612e-2	2	NC	4	NC	1
514		min	0	3	-.111	2	-.034	2	-1.069e-2	3	2376.333	2	7434.622	2
515	11	max	.003	1	.11	3	.029	3	1.486e-2	2	NC	5	NC	2
516		min	0	3	-.192	2	-.016	2	-9.87e-3	3	1165.414	2	8253.683	1
517	12	max	.003	1	.189	3	.105	1	1.359e-2	2	NC	5	NC	5
518		min	0	3	-.371	2	-.003	10	-9.047e-3	3	549.017	2	1694.382	1
519	13	max	.003	1	.275	3	.193	1	1.233e-2	2	NC	5	NC	5
520		min	0	3	-567	2	.009	15	-8.224e-3	3	347.938	2	939.273	1
521	14	max	.003	1	.341	3	.252	1	1.106e-2	2	NC	15	NC	5
522		min	0	3	-.718	2	.012	15	-7.402e-3	3	271.336	2	725.873	1
523	15	max	.004	1	.368	3	.263	1	9.8e-3	2	NC	15	NC	12
524		min	0	3	-.782	2	.012	15	-6.579e-3	3	248.049	2	695.27	1
525	16	max	.004	1	.346	3	.225	1	8.536e-3	2	NC	15	NC	5
526		min	0	3	-.738	2	.011	15	-5.756e-3	3	263.559	2	811.892	1
527	17	max	.004	1	.274	3	.148	1	7.271e-3	2	NC	15	NC	3
528		min	0	3	-.583	2	.007	15	-4.934e-3	3	337.581	2	1220.999	1
529	18	max	.004	1	.161	3	.058	1	6.007e-3	2	NC	5	NC	3
530		min	0	3	-.336	2	.001	10	-4.111e-3	3	613.329	2	2964.032	1
531	19	max	.004	1	.023	3	.009	3	4.742e-3	2	NC	1	NC	1
532		min	0	3	-.032	2	-.009	2	-3.288e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.992e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.539e-5	2	NC	1	NC	1
535	2	max	0	3	-.006	15	.001	1	9.369e-4	3	NC	5	NC	1
536		min	0	10	-.024	4	0	3	-6.325e-4	2	4354.283	4	NC	1
537	3	max	0	3	-.011	15	.004	1	1.475e-3	3	9426.102	15	NC	1
538		min	0	10	-.046	4	-.004	3	-1.2e-3	2	2215.746	4	NC	1
539	4	max	0	3	-.016	15	.008	1	2.012e-3	3	6466.856	15	NC	4
540		min	0	10	-.067	4	-.008	3	-1.767e-3	2	1520.131	4	7093.337	3
541	5	max	0	3	-.02	15	.013	1	2.55e-3	3	5046.154	15	NC	4
542		min	0	10	-.086	4	-.013	3	-2.334e-3	2	1186.174	4	4677.631	3
543	6	max	0	3	-.024	15	.018	1	3.088e-3	3	4246.872	15	NC	4
544		min	0	10	-.103	4	-.019	3	-2.901e-3	2	998.291	4	3417.063	3
545	7	max	0	3	-.027	15	.024	1	3.626e-3	3	3766.209	15	NC	4
546		min	0	10	-.116	4	-.026	3	-3.468e-3	2	885.304	4	2677.623	3
547	8	max	0	3	-.029	15	.029	1	4.163e-3	3	3477.739	15	NC	4
548		min	0	10	-.126	4	-.031	3	-4.035e-3	2	817.495	4	2211.778	3
549	9	max	0	3	-.031	15	.034	1	4.701e-3	3	3322.468	15	NC	4
550		min	0	10	-.132	4	-.037	3	-4.602e-3	2	780.996	4	1906.51	3
551	10	max	0	3	-.031	15	.038	1	5.239e-3	3	3273.347	15	NC	4
552		min	0	10	-.134	4	-.041	3	-5.169e-3	2	769.449	4	1704.918	3
553	11	max	0	3	-.031	15	.04	1	5.776e-3	3	3322.468	15	NC	4
554		min	0	10	-.132	4	-.044	3	-5.736e-3	2	780.996	4	1577.099	3
555	12	max	0	3	-.03	15	.041	1	6.314e-3	3	3477.739	15	NC	5
556		min	0	10	-.126	4	-.045	3	-6.304e-3	2	817.495	4	1508.246	3
557	13	max	0	3	-.027	15	.04	1	6.852e-3	3	3766.209	15	NC	5
558		min	0	10	-.116	4	-.043	3	-6.871e-3	2	885.304	4	1494.167	3
559	14	max	.001	3	-.024	15	.036	1	7.39e-3	3	4246.872	15	NC	4
560		min	0	10	-.103	4	-.039	3	-7.438e-3	2	998.291	4	1541.523	3
561	15	max	.001	3	-.02	15	.03	1	7.927e-3	3	5046.154	15	NC	4
562		min	0	10	-.087	4	-.032	3	-8.005e-3	2	1186.174	4	1674.39	3
563	16	max	.001	3	-.016	15	.021	1	8.465e-3	3	6466.856	15	NC	4
564		min	0	10	-.068	4	-.021	3	-8.572e-3	2	1520.131	4	1957.974	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	-.011	15	.008	1	9.003e-3	3	9426.102	15	NC	4
566		min	-.001	10	-.047	4	-.006	3	-9.139e-3	2	2215.746	4	2596.738	3
567	18	max	.001	3	-.006	15	.014	3	9.541e-3	3	NC	5	NC	4
568		min	-.001	10	-.025	4	-.016	2	-9.706e-3	2	4354.283	4	4624.833	3
569	19	max	.001	3	.005	2	.038	3	1.008e-2	3	NC	1	NC	1
570		min	-.001	10	-.003	9	-.039	2	-1.027e-2	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.011	3	2.984e-3	3	NC	1	NC	1
572		min	-.001	3	-.002	9	-.011	2	-2.786e-3	2	NC	1	NC	1
573	2	max	0	10	-.006	15	.005	1	2.865e-3	3	NC	5	NC	2
574		min	-.001	3	-.024	4	-.002	10	-2.666e-3	2	4354.283	4	9223.654	1
575	3	max	0	10	-.011	15	.014	1	2.746e-3	3	9426.102	15	NC	4
576		min	-.001	3	-.047	4	-.003	3	-2.545e-3	2	2215.746	4	5214.73	1
577	4	max	0	10	-.016	15	.02	1	2.628e-3	3	6466.856	15	NC	4
578		min	-.001	3	-.068	4	-.008	3	-2.425e-3	2	1520.131	4	3962.723	1
579	5	max	0	10	-.02	15	.025	1	2.509e-3	3	5046.154	15	NC	4
580		min	-.001	3	-.087	4	-.011	3	-2.304e-3	2	1186.174	4	3418.949	1
581	6	max	0	10	-.024	15	.028	1	2.39e-3	3	4246.872	15	NC	4
582		min	-.001	3	-.103	4	-.013	3	-2.184e-3	2	998.291	4	3179.844	1
583	7	max	0	10	-.027	15	.029	1	2.272e-3	3	3766.209	15	NC	4
584		min	0	3	-.116	4	-.014	3	-2.064e-3	2	885.304	4	3118.756	1
585	8	max	0	10	-.029	15	.028	1	2.153e-3	3	3477.739	15	NC	4
586		min	0	3	-.126	4	-.014	3	-1.943e-3	2	817.495	4	3192.074	1
587	9	max	0	10	-.031	15	.027	1	2.035e-3	3	3322.468	15	NC	4
588		min	0	3	-.131	4	-.013	3	-1.823e-3	2	780.996	4	3393.333	1
589	10	max	0	10	-.031	15	.024	1	1.916e-3	3	3273.347	15	NC	4
590		min	0	3	-.133	4	-.012	3	-1.702e-3	2	769.449	4	3742.397	1
591	11	max	0	10	-.031	15	.021	1	1.797e-3	3	3322.468	15	NC	4
592		min	0	3	-.131	4	-.01	3	-1.582e-3	2	780.996	4	4289.57	1
593	12	max	0	10	-.029	15	.017	1	1.679e-3	3	3477.739	15	NC	4
594		min	0	3	-.125	4	-.008	3	-1.462e-3	2	817.495	4	5134.789	1
595	13	max	0	10	-.027	15	.014	1	1.56e-3	3	3766.209	15	NC	3
596		min	0	3	-.116	4	-.006	3	-1.341e-3	2	885.304	4	6476.83	1
597	14	max	0	10	-.024	15	.01	1	1.441e-3	3	4246.872	15	NC	2
598		min	0	3	-.103	4	-.004	3	-1.221e-3	2	998.291	4	8743.387	1
599	15	max	0	10	-.02	15	.006	1	1.323e-3	3	5046.154	15	NC	1
600		min	0	3	-.086	4	-.002	3	-1.1e-3	2	1186.174	4	NC	1
601	16	max	0	10	-.016	15	.003	1	1.204e-3	3	6466.856	15	NC	1
602		min	0	3	-.067	4	0	3	-9.8e-4	2	1520.131	4	NC	1
603	17	max	0	10	-.011	15	.001	9	1.086e-3	3	9426.102	15	NC	1
604		min	0	3	-.046	4	0	10	-8.596e-4	2	2215.746	4	NC	1
605	18	max	0	10	-.006	15	0	3	9.67e-4	3	NC	5	NC	1
606		min	0	3	-.024	4	0	2	-7.392e-4	2	4354.283	4	NC	1
607	19	max	0	1	0	1	0	1	8.484e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-6.188e-4	2	NC	1	NC	1



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.