

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

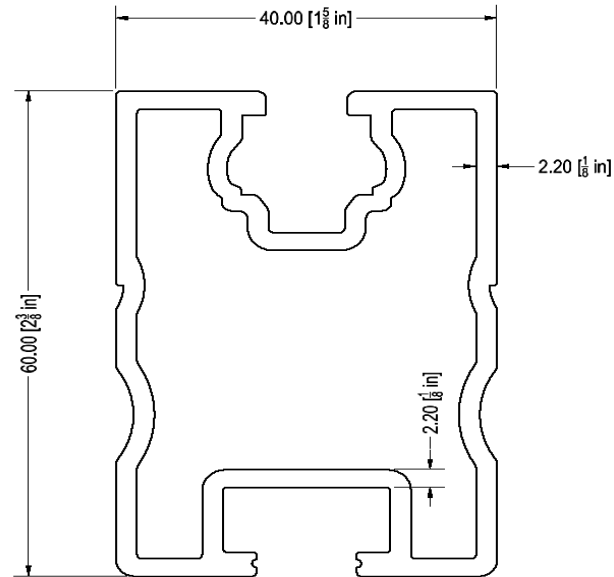
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	63 in
ΦF_{ty} STRONG-AXIS =	29.20 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.468 k-ft
M_z =	0.113 k-ft
$M_{y \text{ allowable}}$ =	1.243 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	51%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.41 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.557 k-ft
M_z =	0.000 k-ft
P_n =	0.287 k
$M_{y \text{ allowable}}$ =	1.443 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	41%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.035 k-ft
P_n =	0.183 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	10%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.603 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	16%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.741 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.004 k-ft
P_n =	0.199 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	10%



A cross brace kit is required every 19 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

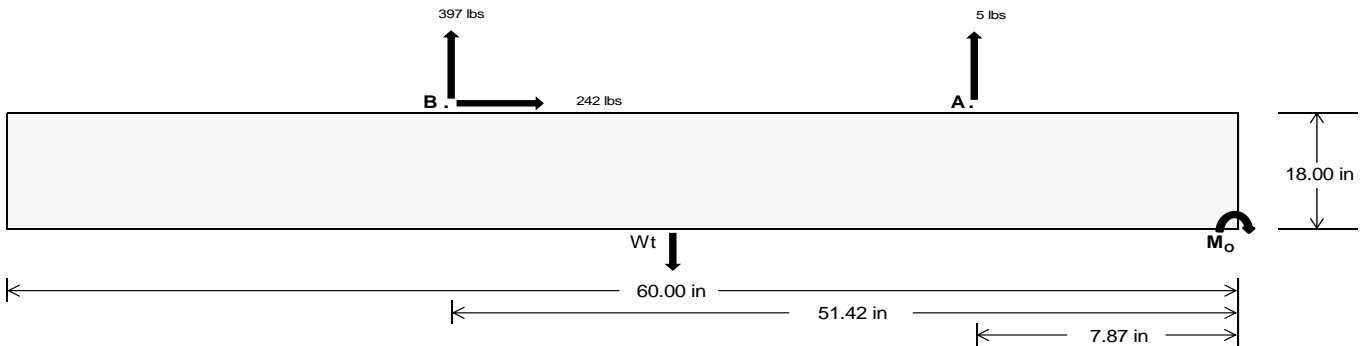
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	28.47	1723.35	k
Compressive Load =	1162.78	1203.19	k
Lateral Load =	28.90	1047.71	k
Moment (Weak Axis) =	0.05	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24789.7$ in-lbs
Resisting Force Required = 826.32 lbs
S.F. = 1.67
Weight Required = 1377.21 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 241.65 lbs
Friction = 0.4
Weight Required = 604.12 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 241.65 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width	20 in	21 in	22 in	23 in
	1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	418 lbs	418 lbs	418 lbs	418 lbs	383 lbs	383 lbs	383 lbs	383 lbs	563 lbs	563 lbs	563 lbs	563 lbs	-10 lbs	-10 lbs	-10 lbs	-10 lbs
F_B	289 lbs	289 lbs	289 lbs	289 lbs	500 lbs	500 lbs	500 lbs	500 lbs	564 lbs	564 lbs	564 lbs	564 lbs	-794 lbs	-794 lbs	-794 lbs	-794 lbs
F_V	45 lbs	45 lbs	45 lbs	45 lbs	437 lbs	437 lbs	437 lbs	437 lbs	358 lbs	358 lbs	358 lbs	358 lbs	-483 lbs	-483 lbs	-483 lbs	-483 lbs
P_{total}	2520 lbs	2610 lbs	2701 lbs	2792 lbs	2696 lbs	2786 lbs	2877 lbs	2968 lbs	2940 lbs	3031 lbs	3121 lbs	3212 lbs	284 lbs	338 lbs	393 lbs	447 lbs
M	324 lbs-ft	324 lbs-ft	324 lbs-ft	324 lbs-ft	469 lbs-ft	469 lbs-ft	469 lbs-ft	469 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	673 lbs-ft	673 lbs-ft	673 lbs-ft	673 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.37 ft	1.99 ft	1.71 ft	1.50 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	255.7 psf	253.9 psf	252.3 psf	250.7 psf	255.9 psf	254.1 psf	252.4 psf	250.9 psf	271.1 psf	268.5 psf	266.2 psf	264.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	349.0 psf	342.7 psf	337.0 psf	331.8 psf	391.1 psf	382.8 psf	375.3 psf	368.4 psf	434.5 psf	424.2 psf	414.8 psf	406.2 psf	870.5 psf	252.0 psf	181.4 psf	156.2 psf

Maximum Bearing Pressure = 870 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

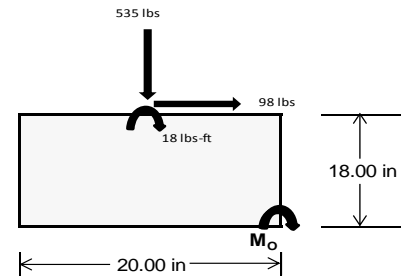
Overturning Check

$M_o = 280.4 \text{ ft-lbs}$
 Resisting Force Required = 336.47 lbs
 S.F. = 1.67
 Weight Required = 560.78 lbs
 Minimum Width = 20 in in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	127 lbs	93 lbs	67 lbs	255 lbs	535 lbs	208 lbs	82 lbs	-21 lbs	23 lbs
F_v	16 lbs	130 lbs	16 lbs	11 lbs	98 lbs	13 lbs	16 lbs	130 lbs	16 lbs
P_{total}	2371 lbs	2337 lbs	2310 lbs	2391 lbs	2671 lbs	2344 lbs	738 lbs	635 lbs	679 lbs
M	46 lbs-ft	220 lbs-ft	47 lbs-ft	32 lbs-ft	165 lbs-ft	37 lbs-ft	46 lbs-ft	219 lbs-ft	47 lbs-ft
e	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.06 ft	0.35 ft	0.07 ft
$L/6$	0.28 ft	1.48 ft	1.63 ft	1.64 ft	1.54 ft	1.64 ft	1.54 ft	0.98 ft	1.53 ft
f_{min}	264.7 sqft	185.7 sqft	256.8 sqft	273.3 sqft	249.0 sqft	265.4 sqft	68.5 sqft	-18.5 sqft	61.1 sqft
f_{max}	304.4 psf	375.3 psf	297.7 psf	300.6 psf	392.0 psf	297.2 psf	108.5 psf	171.0 psf	101.9 psf



Maximum Bearing Pressure = 392 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

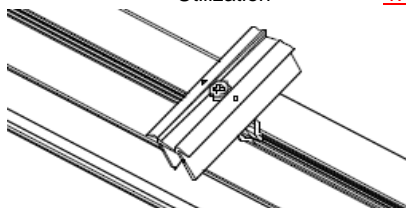
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

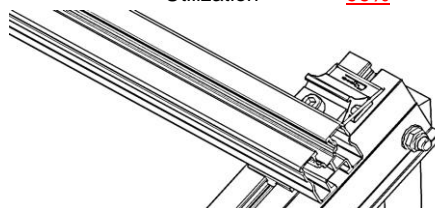
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.571 k
Allowable Uplift =	1.214 k
Utilization =	<u>47%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.067 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

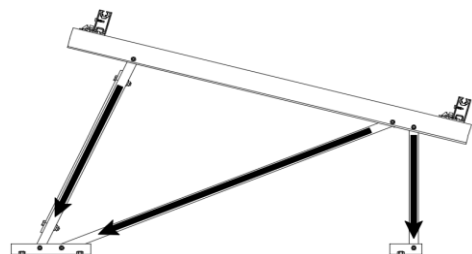
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.894 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

Diagonal Strut

Maximum Axial Load =	0.603 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



Rear Strut

Maximum Axial Load =	1.114 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.199 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	$0.020h_{sx}$
	0.646 in
Max Drift, Δ_{MAX} =	0.073 in
	<u>$0.073 \leq 0.646$. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$164.048$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$170.354$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.1$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.2 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.243 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.10 \\
 &23.4092 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.10 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.443 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



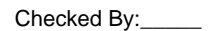
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	233.465	1	-0.016	15	.271	1	0	10	0	4	0	15
30			min	-350.07	3	-0.072	3	-.483	5	0	4	0	3	0	6
31		16	max	233.591	1	-.028	15	.271	1	0	10	0	4	0	15
32			min	-349.975	3	-.115	4	-.597	5	0	4	0	3	0	6
33		17	max	233.717	1	-.04	15	.271	1	0	10	0	4	0	15
34			min	-349.881	3	-.166	4	-.712	5	0	4	0	3	0	6
35		18	max	233.843	1	-.052	15	.271	1	0	10	0	1	0	15
36			min	-349.786	3	-.217	4	-.826	5	0	4	0	3	0	6
37		19	max	233.968	1	-.064	15	.271	1	0	10	0	1	0	15
38			min	-349.692	3	-.268	4	-.94	5	0	4	0	3	0	6
39	M3	1	max	165.618	2	1.756	6	-.022	10	0	5	0	1	0	6
40			min	-169.057	3	.412	15	-1.36	4	0	1	0	10	0	15
41		2	max	165.549	2	1.579	6	-.022	10	0	5	0	1	0	2
42			min	-169.109	3	.371	15	-1.227	4	0	1	0	10	0	12
43		3	max	165.479	2	1.403	6	-.022	10	0	5	0	1	0	2
44			min	-169.161	3	.329	15	-1.093	4	0	1	0	5	0	3
45		4	max	165.41	2	1.226	6	-.022	10	0	5	0	1	0	15
46			min	-169.213	3	.287	15	-.959	4	0	1	0	5	0	4
47		5	max	165.341	2	1.049	6	-.022	10	0	5	0	1	0	15
48			min	-169.265	3	.246	15	-.826	4	0	1	0	5	0	4
49		6	max	165.271	2	.872	6	-.022	10	0	5	0	1	0	15
50			min	-169.317	3	.204	15	-.692	4	0	1	0	5	0	4
51		7	max	165.202	2	.695	6	-.022	10	0	5	0	1	0	15
52			min	-169.369	3	.163	15	-.558	4	0	1	0	5	0	4
53		8	max	165.133	2	.518	6	-.022	10	0	5	0	1	0	15
54			min	-169.421	3	.121	15	-.425	4	0	1	0	5	-.001	4
55		9	max	165.063	2	.342	6	-.022	10	0	5	0	1	0	15
56			min	-169.473	3	.08	15	-.291	4	0	1	0	5	-.001	4
57		10	max	164.994	2	.165	6	-.022	10	0	5	0	1	0	15
58			min	-169.525	3	.038	15	-.286	1	0	1	0	5	-.001	4
59		11	max	164.925	2	.017	2	.041	5	0	5	0	1	0	15
60			min	-169.577	3	-.037	3	-.286	1	0	1	0	5	-.001	4
61		12	max	164.855	2	-.045	15	.174	5	0	5	0	1	0	15
62			min	-169.629	3	-.189	4	-.286	1	0	1	0	5	-.001	4
63		13	max	164.786	2	-.087	15	.308	5	0	5	0	1	0	15
64			min	-169.681	3	-.366	4	-.286	1	0	1	0	5	-.001	4
65		14	max	164.717	2	-.128	15	.442	5	0	5	0	1	0	15
66			min	-169.733	3	-.543	4	-.286	1	0	1	0	5	-.001	4
67		15	max	164.647	2	-.17	15	.575	5	0	5	0	1	0	15
68			min	-169.785	3	-.72	4	-.286	1	0	1	0	5	0	4
69		16	max	164.578	2	-.211	15	.709	5	0	5	0	1	0	15
70			min	-169.837	3	-.896	4	-.286	1	0	1	0	5	0	4
71		17	max	164.509	2	-.253	15	.843	5	0	5	0	10	0	15
72			min	-169.889	3	-1.073	4	-.286	1	0	1	0	4	0	4
73		18	max	164.439	2	-.294	15	.976	5	0	5	0	10	0	15
74			min	-169.941	3	-1.25	4	-.286	1	0	1	0	4	0	4
75		19	max	164.37	2	-.336	15	1.11	5	0	5	0	5	0	1
76			min	-169.993	3	-1.427	4	-.286	1	0	1	0	1	0	1
77	M4	1	max	332.039	1	0	1	-.087	10	0	1	0	5	0	1
78			min	3.531	12	0	1	-21.186	4	0	1	0	2	0	1
79		2	max	332.104	1	0	1	-.087	10	0	1	0	12	0	1
80			min	3.564	12	0	1	-21.242	4	0	1	-.002	4	0	1
81		3	max	332.168	1	0	1	-.087	10	0	1	0	12	0	1
82			min	3.596	12	0	1	-21.299	4	0	1	-.004	4	0	1
83		4	max	332.233	1	0	1	-.087	10	0	1	0	10	0	1
84			min	3.628	12	0	1	-21.355	4	0	1	-.006	4	0	1
85		5	max	332.298	1	0	1	-.087	10	0	1	0	10	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	740.463	1	.047	2	.075	9	0	3	0	4	0	12
144		min	-1112.596	3	-.148	3	-.515	5	0	5	0	3	0	2
145	16	max	740.589	1	.007	2	.075	9	0	3	0	4	0	12
146		min	-1112.502	3	-.178	3	-.63	5	0	5	0	3	0	2
147	17	max	740.715	1	-.033	2	.075	9	0	3	0	4	0	12
148		min	-1112.407	3	-.208	3	-.744	5	0	5	0	3	0	2
149	18	max	740.841	1	-.061	15	.075	9	0	3	0	4	0	3
150		min	-1112.313	3	-.238	3	-.859	5	0	5	0	3	0	2
151	19	max	740.967	1	-.073	15	.075	9	0	3	0	4	0	3
152		min	-1112.219	3	-.281	4	-.973	5	0	5	0	3	0	2
153	M7	1	max	603.439	2	1.774	.026	3	0	1	0	4	0	2
154		min	-511.085	3	.423	15	-1.329	4	0	3	0	3	0	3
155	2	max	603.369	2	1.597	4	.026	3	0	1	0	4	0	2
156		min	-511.137	3	.381	15	-1.195	4	0	3	0	3	0	3
157	3	max	603.3	2	1.421	4	.026	3	0	1	0	1	0	2
158		min	-511.189	3	.34	15	-1.061	4	0	3	0	3	0	3
159	4	max	603.231	2	1.244	4	.026	3	0	1	0	1	0	2
160		min	-511.241	3	.298	15	-.928	4	0	3	0	3	0	3
161	5	max	603.161	2	1.067	4	.026	3	0	1	0	1	0	15
162		min	-511.293	3	.257	15	-.794	4	0	3	0	5	0	3
163	6	max	603.092	2	.89	4	.026	3	0	1	0	1	0	15
164		min	-511.345	3	.215	15	-.66	4	0	3	0	5	0	6
165	7	max	603.023	2	.713	4	.026	3	0	1	0	1	0	15
166		min	-511.397	3	.173	15	-.527	4	0	3	0	5	0	6
167	8	max	602.953	2	.536	4	.026	3	0	1	0	1	0	15
168		min	-511.449	3	.132	15	-.393	4	0	3	0	5	-.001	6
169	9	max	602.884	2	.36	4	.026	3	0	1	0	1	0	15
170		min	-511.501	3	.075	12	-.259	4	0	3	0	5	-.001	6
171	10	max	602.815	2	.215	2	.026	3	0	1	0	1	0	15
172		min	-511.553	3	-.001	3	-.126	4	0	3	0	5	-.001	6
173	11	max	602.745	2	.078	2	.026	3	0	1	0	1	0	15
174		min	-511.605	3	-.105	3	-.015	2	0	3	0	5	-.001	6
175	12	max	602.676	2	-.034	15	.142	5	0	1	0	1	0	15
176		min	-511.657	3	-.208	3	-.015	2	0	3	0	5	-.001	6
177	13	max	602.607	2	-.076	15	.275	5	0	1	0	1	0	15
178		min	-511.709	3	-.348	6	-.015	2	0	3	0	5	-.001	6
179	14	max	602.537	2	-.118	15	.409	5	0	1	0	1	0	15
180		min	-511.761	3	-.525	6	-.015	2	0	3	0	5	-.001	6
181	15	max	602.468	2	-.159	15	.543	5	0	1	0	1	0	15
182		min	-511.813	3	-.702	6	-.015	2	0	3	0	5	0	6
183	16	max	602.399	2	-.201	15	.676	5	0	1	0	1	0	15
184		min	-511.865	3	-.879	6	-.015	2	0	3	0	5	0	6
185	17	max	602.33	2	-.242	15	.81	5	0	1	0	1	0	15
186		min	-511.917	3	-1.056	6	-.015	2	0	3	0	5	0	6
187	18	max	602.26	2	-.284	15	.944	5	0	1	0	1	0	15
188		min	-511.969	3	-1.232	6	-.015	2	0	3	0	5	0	6
189	19	max	602.191	2	-.325	15	1.077	5	0	1	0	1	0	1
190		min	-512.021	3	-1.409	6	-.015	2	0	3	0	3	0	1
191	M8	1	max	893.279	1	0	.479	1	0	1	0	4	0	1
192		min	-22.77	3	0	1	-21.411	4	0	1	0	1	0	1
193	2	max	893.344	1	0	1	.479	1	0	1	0	1	0	1
194		min	-22.722	3	0	1	-21.467	4	0	1	-.002	4	0	1
195	3	max	893.409	1	0	1	.479	1	0	1	0	1	0	1
196		min	-22.673	3	0	1	-21.523	4	0	1	-.004	4	0	1
197	4	max	893.474	1	0	1	.479	1	0	1	0	1	0	1
198		min	-22.625	3	0	1	-21.579	4	0	1	-.006	4	0	1
199	5	max	893.538	1	0	1	.479	1	0	1	0	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	235.842	1	.006	5	-.006	12	0	1	.001	5	0	15
258			min	-308.687	3	-.052	9	-.372	4	-.001	5	0	3	0	4
259		16	max	235.968	1	-.007	15	-.006	12	0	1	0	5	0	15
260			min	-308.593	3	-.085	9	-.486	4	-.001	5	0	3	0	4
261		17	max	236.093	1	-.019	15	-.006	12	0	1	0	5	0	15
262			min	-308.498	3	-.136	6	-.601	4	-.001	5	0	3	0	4
263		18	max	236.219	1	-.031	15	-.006	12	0	1	0	5	0	15
264			min	-308.404	3	-.187	6	-.715	4	-.001	5	0	1	0	4
265		19	max	236.345	1	-.043	15	-.006	12	0	1	0	5	0	15
266			min	-308.309	3	-.239	6	-.83	4	-.001	5	0	1	0	4
267	M11	1	max	165.183	2	1.747	6	.312	1	.001	4	0	5	0	6
268			min	-169.723	3	.405	15	-1.241	5	0	10	0	1	0	15
269		2	max	165.113	2	1.57	6	.312	1	.001	4	0	5	0	2
270			min	-169.775	3	.364	15	-1.107	5	0	10	0	1	0	3
271		3	max	165.044	2	1.393	6	.312	1	.001	4	0	5	0	2
272			min	-169.827	3	.322	15	-.974	5	0	10	0	1	0	3
273		4	max	164.975	2	1.216	6	.312	1	.001	4	0	3	0	15
274			min	-169.879	3	.281	15	-.84	5	0	10	0	1	0	4
275		5	max	164.905	2	1.039	6	.312	1	.001	4	0	3	0	15
276			min	-169.931	3	.239	15	-.706	5	0	10	0	1	0	4
277		6	max	164.836	2	.863	6	.312	1	.001	4	0	3	0	15
278			min	-169.983	3	.198	15	-.573	5	0	10	0	1	0	4
279		7	max	164.767	2	.686	6	.312	1	.001	4	0	3	0	15
280			min	-170.035	3	.156	15	-.439	5	0	10	0	1	0	4
281		8	max	164.697	2	.509	6	.312	1	.001	4	0	3	0	15
282			min	-170.087	3	.115	15	-.305	5	0	10	0	1	-.001	4
283		9	max	164.628	2	.332	6	.312	1	.001	4	0	3	0	15
284			min	-170.139	3	.073	15	-.172	5	0	10	0	4	-.001	4
285		10	max	164.559	2	.155	6	.312	1	.001	4	0	3	0	15
286			min	-170.191	3	.031	15	-.038	5	0	10	0	4	-.001	4
287		11	max	164.489	2	.017	2	.312	1	.001	4	0	3	0	15
288			min	-170.243	3	-.051	3	-.033	3	0	10	0	4	-.001	4
289		12	max	164.42	2	-.052	15	.312	1	.001	4	0	3	0	15
290			min	-170.295	3	-.199	4	-.033	3	0	10	0	4	-.001	4
291		13	max	164.351	2	-.093	15	.433	4	.001	4	0	3	0	15
292			min	-170.347	3	-.376	4	-.033	3	0	10	0	4	-.001	4
293		14	max	164.282	2	-.135	15	.567	4	.001	4	0	3	0	15
294			min	-170.399	3	-.553	4	-.033	3	0	10	0	4	-.001	4
295		15	max	164.212	2	-.176	15	.701	4	.001	4	0	3	0	15
296			min	-170.451	3	-.729	4	-.033	3	0	10	0	5	0	4
297		16	max	164.143	2	-.218	15	.834	4	.001	4	0	3	0	15
298			min	-170.503	3	-.906	4	-.033	3	0	10	0	10	0	4
299		17	max	164.074	2	-.26	15	.968	4	.001	4	0	4	0	15
300			min	-170.555	3	-1.083	4	-.033	3	0	10	0	10	0	4
301		18	max	164.004	2	-.301	15	1.102	4	.001	4	0	4	0	15
302			min	-170.607	3	-1.26	4	-.033	3	0	10	0	10	0	4
303		19	max	163.935	2	-.343	15	1.235	4	.001	4	0	4	0	1
304			min	-170.659	3	-1.437	4	-.033	3	0	10	0	10	0	1
305	M12	1	max	331.889	1	0	1	1.841	1	0	1	0	4	0	1
306			min	3.764	15	0	1	-19.624	5	0	1	0	3	0	1
307		2	max	331.954	1	0	1	1.841	1	0	1	0	1	0	1
308			min	3.783	15	0	1	-19.68	5	0	1	-.002	5	0	1
309		3	max	332.019	1	0	1	1.841	1	0	1	0	1	0	1
310			min	3.803	15	0	1	-19.736	5	0	1	-.003	5	0	1
311		4	max	332.083	1	0	1	1.841	1	0	1	0	1	0	1
312			min	3.822	15	0	1	-19.792	5	0	1	-.005	5	0	1
313		5	max	332.148	1	0	1	1.841	1	0	1	0	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	86.844	3	2.931	9	-2.622	10	0	12	-.002	12	.163	2
372			min	-10.64	10	-26.097	2	-36.863	1	0	1	-.04	1	-.106	3
373		16	max	88.314	2	110.777	2	-2.642	10	0	1	-.003	12	.168	2
374			min	-5.67	3	-157.712	3	-37.11	1	0	5	-.048	1	-.102	3
375		17	max	88.453	2	110.535	2	-2.642	10	0	1	-.004	12	.144	2
376			min	-5.565	3	-157.893	3	-37.11	1	0	5	-.056	1	-.067	3
377		18	max	-4.835	12	332.945	2	-2.729	10	0	3	-.004	12	.072	2
378			min	-104.035	1	-154.77	3	-38.033	1	0	2	-.064	1	-.034	3
379		19	max	-4.765	12	332.703	2	-2.729	10	0	3	-.005	12	0	2
380			min	-103.896	1	-154.952	3	-38.033	1	0	2	-.073	1	0	3
381	M5	1	max	238.413	1	1080.667	3	0	10	0	1	.035	4	0	3
382			min	5.54	12	-757.675	1	-58.2	3	0	5	0	10	0	2
383		2	max	238.552	1	1080.485	3	0	10	0	1	.03	4	.164	1
384			min	5.61	12	-757.917	1	-58.2	3	0	5	-.005	3	-.234	3
385		3	max	256.634	3	5.379	9	6.518	3	0	3	.025	4	.325	1
386			min	-45.508	2	-83.75	2	-19.121	4	0	4	-.017	3	-.463	3
387		4	max	256.739	3	5.178	9	6.518	3	0	3	.021	4	.339	2
388			min	-45.369	2	-83.992	2	-18.879	4	0	4	-.016	3	-.453	3
389		5	max	256.843	3	4.976	9	6.518	3	0	3	.017	4	.358	2
390			min	-45.229	2	-84.234	2	-18.637	4	0	4	-.014	3	-.442	3
391		6	max	256.948	3	4.775	9	6.518	3	0	3	.013	4	.376	2
392			min	-45.09	2	-84.476	2	-18.395	4	0	4	-.013	3	-.432	3
393		7	max	257.053	3	4.573	9	6.518	3	0	3	.009	4	.394	2
394			min	-44.95	2	-84.718	2	-18.153	4	0	4	-.012	3	-.421	3
395		8	max	257.158	3	4.372	9	6.518	3	0	3	.005	4	.413	2
396			min	-44.81	2	-84.959	2	-17.911	4	0	4	-.01	3	-.41	3
397		9	max	257.262	3	4.17	9	6.518	3	0	3	0	4	.431	2
398			min	-44.671	2	-85.201	2	-17.669	4	0	4	-.009	3	-.4	3
399		10	max	257.367	3	3.969	9	6.518	3	0	3	0	2	.45	2
400			min	-44.531	2	-85.443	2	-17.427	4	0	4	-.007	3	-.389	3
401		11	max	257.472	3	3.767	9	6.518	3	0	3	0	10	.468	2
402			min	-44.391	2	-85.685	2	-17.185	4	0	4	-.007	4	-.378	3
403		12	max	257.576	3	3.565	9	6.518	3	0	3	0	10	.487	2
404			min	-44.252	2	-85.927	2	-16.943	4	0	4	-.01	4	-.367	3
405		13	max	257.681	3	3.364	9	6.518	3	0	3	0	10	.505	2
406			min	-44.112	2	-86.168	2	-16.701	4	0	4	-.014	4	-.357	3
407		14	max	257.786	3	3.162	9	6.518	3	0	3	0	10	.524	2
408			min	-43.973	2	-86.41	2	-16.459	4	0	4	-.018	4	-.346	3
409		15	max	257.891	3	2.961	9	6.518	3	0	3	0	10	.543	2
410			min	-43.833	2	-86.652	2	-16.217	4	0	4	-.021	4	-.335	3
411		16	max	287.402	2	419.363	2	6.491	3	0	3	0	3	.557	2
412			min	-22.528	3	-479.019	3	-14.905	4	0	4	-.025	4	-.32	3
413		17	max	287.542	2	419.121	2	6.491	3	0	3	.002	3	.466	2
414			min	-22.424	3	-479.2	3	-14.663	4	0	4	-.028	4	-.216	3
415		18	max	-7.506	12	1082.726	2	5.95	3	0	4	.003	3	.234	2
416			min	-238.575	1	-499.203	3	-35.28	5	0	1	-.035	4	-.108	3
417		19	max	-7.436	12	1082.484	2	5.95	3	0	4	.005	3	0	3
418			min	-238.435	1	-499.385	3	-35.038	5	0	1	-.043	4	0	2
419	M9	1	max	103.563	1	330.62	3	148.625	4	0	3	0	15	0	2
420			min	2.225	15	-232.167	1	2.637	10	0	1	-.072	1	0	3
421		2	max	103.702	1	330.439	3	148.867	4	0	3	.029	5	.051	1
422			min	2.267	15	-232.408	1	2.637	10	0	1	-.064	1	-.072	3
423		3	max	85.473	3	5.326	9	36.031	1	0	1	.058	5	.1	1
424			min	-11.611	10	-23.202	2	-24.466	5	0	5	-.055	1	-.142	3
425		4	max	85.577	3	5.125	9	36.031	1	0	1	.052	5	.104	2
426			min	-11.495	10	-23.444	2	-24.224	5	0	5	-.047	1	-.139	3
427		5	max	85.682	3	4.923	9	36.031	1	0	1	.047	5	.109	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-11.379	10	-23.686	2	-23.982	5	0	5	-.039	1	-.137	3
429	6	max	85.787	3	4.722	9	36.031	1	0	1	.042	5	.114	2
430		min	-11.262	10	-23.928	2	-23.74	5	0	5	-.031	1	-.134	3
431	7	max	85.892	3	4.52	9	36.031	1	0	1	.037	5	.12	2
432		min	-11.146	10	-24.17	2	-23.498	5	0	5	-.023	1	-.131	3
433	8	max	85.996	3	4.319	9	36.031	1	0	1	.032	5	.125	2
434		min	-11.03	10	-24.412	2	-23.256	5	0	5	-.016	1	-.128	3
435	9	max	86.101	3	4.117	9	36.031	1	0	1	.027	5	.13	2
436		min	-10.913	10	-24.653	2	-23.014	5	0	5	-.008	1	-.125	3
437	10	max	86.206	3	3.916	9	36.031	1	0	1	.022	4	.136	2
438		min	-10.797	10	-24.895	2	-22.772	5	0	5	0	1	-.122	3
439	11	max	86.31	3	3.714	9	36.031	1	0	1	.019	4	.141	2
440		min	-10.681	10	-25.137	2	-22.53	5	0	5	0	10	-.119	3
441	12	max	86.415	3	3.513	9	36.031	1	0	1	.016	1	.147	2
442		min	-10.564	10	-25.379	2	-22.288	5	0	5	.001	10	-.116	3
443	13	max	86.52	3	3.311	9	36.031	1	0	1	.023	1	.152	2
444		min	-10.448	10	-25.621	2	-22.046	5	0	5	.002	10	-.113	3
445	14	max	86.625	3	3.109	9	36.031	1	0	1	.031	1	.158	2
446		min	-10.332	10	-25.862	2	-21.804	5	0	5	.002	15	-.109	3
447	15	max	86.729	3	2.908	9	36.031	1	0	1	.039	1	.163	2
448		min	-10.215	10	-26.104	2	-21.562	5	0	5	-.002	5	-.106	3
449	16	max	88.55	2	110.406	2	36.303	1	0	10	.047	1	.168	2
450		min	-6.138	3	-158.19	3	-20.157	5	0	4	-.006	5	-.102	3
451	17	max	88.689	2	110.165	2	36.303	1	0	10	.055	1	.144	2
452		min	-6.033	3	-158.371	3	-19.915	5	0	4	-.01	5	-.067	3
453	18	max	3.914	5	332.945	2	38.163	1	0	2	.063	1	.072	2
454		min	-103.694	1	-154.764	3	-39.269	5	0	3	-.019	5	-.034	3
455	19	max	3.979	5	332.703	2	38.163	1	0	2	.072	1	0	2
456		min	-103.554	1	-154.945	3	-39.027	5	0	3	-.027	5	0	3
457	M13	1	max	148.628	4	231.89	1	-2.225	15	0	.072	1	0	1
458		min	2.638	10	-330.641	3	-103.554	1	0	3	0	15	0	3
459	2	max	142.899	4	164.165	1	-1.228	15	0	2	.019	1	.165	3
460		min	2.638	10	-233.874	3	-78.783	1	0	3	0	5	-.116	1
461	3	max	137.17	4	96.439	1	-.231	15	0	2	.007	3	.273	3
462		min	2.638	10	-137.107	3	-54.011	1	0	3	-.02	1	-.192	1
463	4	max	131.44	4	28.733	2	1.07	5	0	2	.003	3	.325	3
464		min	2.638	10	-40.341	3	-29.24	1	0	3	-.044	1	-.228	1
465	5	max	125.711	4	56.426	3	2.612	5	0	2	.001	3	.32	3
466		min	2.638	10	-39.012	1	-4.468	1	0	3	-.054	1	-.225	1
467	6	max	119.982	4	153.192	3	20.303	1	0	2	.002	5	.259	3
468		min	2.638	10	-106.737	1	-1.705	3	0	3	-.05	1	-.183	1
469	7	max	114.253	4	249.959	3	45.075	1	0	2	.005	5	.141	3
470		min	2.638	10	-174.462	1	-.254	3	0	3	-.031	1	-.101	2
471	8	max	108.523	4	346.726	3	69.846	1	0	2	.009	4	.021	1
472		min	2.638	10	-242.188	1	.987	12	0	3	0	3	-.033	3
473	9	max	102.794	4	443.492	3	94.618	1	0	2	.051	1	.182	1
474		min	2.638	10	-309.913	1	1.954	12	0	3	0	12	-.263	3
475	10	max	97.065	4	540.259	3	119.389	1	0	2	.113	1	.383	1
476		min	2.638	10	-377.639	1	2.922	12	0	3	-.005	3	-.55	3
477	11	max	69.18	4	309.913	1	2.548	5	0	3	.05	1	.182	1
478		min	2.638	10	-443.492	3	-94.27	1	0	2	-.014	5	-.263	3
479	12	max	63.451	4	242.188	1	4.09	5	0	3	.004	2	.021	1
480		min	2.638	10	-346.726	3	-69.498	1	0	2	-.012	5	-.033	3
481	13	max	57.722	4	174.462	1	5.633	5	0	3	-.002	10	.141	3
482		min	2.638	10	-249.959	3	-44.727	1	0	2	-.031	1	-.101	2
483	14	max	51.992	4	106.737	1	7.175	5	0	3	-.004	15	.259	3
484		min	2.638	10	-153.192	3	-19.955	1	0	2	-.05	1	-.183	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	46.263	4	39.012	1	10.082	4	0	3	0	15	.32	3
486			min	2.638	10	-56.426	3	-.647	10	0	2	-.054	1	-.225	1
487		16	max	40.534	4	40.341	3	29.588	1	0	3	.005	5	.325	3
488			min	2.638	10	-28.733	2	2.107	10	0	2	-.044	1	-.228	1
489		17	max	37.073	1	137.107	3	54.359	1	0	3	.011	5	.273	3
490			min	2.638	10	-96.439	1	4.16	12	0	2	-.02	1	-.192	1
491		18	max	37.073	1	233.874	3	79.13	1	0	3	.023	4	.165	3
492			min	2.638	10	-164.165	1	5.128	12	0	2	0	10	-.116	1
493		19	max	37.073	1	330.641	3	103.902	1	0	3	.073	1	0	1
494			min	2.638	10	-231.89	1	6.095	12	0	2	.005	10	0	3
495	M16	1	max	39.019	5	332.856	2	3.979	5	0	3	.072	1	0	2
496			min	-38.068	1	-154.97	3	-103.563	1	0	2	-.027	5	0	3
497		2	max	33.29	5	235.657	2	5.521	5	0	3	.019	1	.077	3
498			min	-38.068	1	-110.002	3	-78.792	1	0	2	-.024	5	-.166	2
499		3	max	27.56	5	138.458	2	7.064	5	0	3	0	12	.128	3
500			min	-38.068	1	-65.033	3	-54.021	1	0	2	-.025	4	-.275	2
501		4	max	21.831	5	41.259	2	8.606	5	0	3	-.002	12	.153	3
502			min	-38.068	1	-20.065	3	-29.249	1	0	2	-.044	1	-.327	2
503		5	max	16.102	5	24.903	3	10.149	5	0	3	-.003	12	.152	3
504			min	-38.068	1	-55.941	2	-4.478	1	0	2	-.054	1	-.323	2
505		6	max	10.372	5	69.872	3	20.294	1	0	3	-.003	15	.124	3
506			min	-38.068	1	-153.14	2	-.749	3	0	2	-.05	1	-.262	2
507		7	max	4.643	5	114.84	3	45.065	1	0	3	.003	5	.07	3
508			min	-38.068	1	-250.339	2	.593	12	0	2	-.031	1	-.144	2
509		8	max	.921	3	159.809	3	69.837	1	0	3	.012	4	.03	2
510			min	-38.068	1	-347.538	2	1.56	12	0	2	-.005	3	-.01	3
511		9	max	.921	3	204.777	3	94.608	1	0	3	.051	1	.261	2
512			min	-38.068	1	-444.738	2	2.528	12	0	2	-.003	3	-.116	3
513		10	max	22.509	5	-9.624	15	119.38	1	0	14	.113	1	.549	2
514			min	-38.068	1	-541.937	2	-5.803	3	0	2	.003	12	-.249	3
515		11	max	16.78	5	444.738	2	2.131	5	0	2	.05	1	.261	2
516			min	-37.943	1	-204.777	3	-94.267	1	0	3	-.011	5	-.116	3
517		12	max	11.05	5	347.538	2	3.673	5	0	2	.004	2	.03	2
518			min	-37.943	1	-159.809	3	-69.496	1	0	3	-.01	5	-.01	3
519		13	max	5.321	5	250.339	2	5.216	5	0	2	-.001	12	.07	3
520			min	-37.943	1	-114.84	3	-44.724	1	0	3	-.031	1	-.144	2
521		14	max	-.213	15	153.14	2	6.758	5	0	2	-.002	12	.124	3
522			min	-37.943	1	-69.872	3	-19.953	1	0	3	-.05	1	-.262	2
523		15	max	-2.729	10	55.941	2	9.642	4	0	2	0	5	.152	3
524			min	-37.943	1	-24.903	3	-.636	10	0	3	-.054	1	-.323	2
525		16	max	-2.729	10	20.065	3	29.59	1	0	2	.006	5	.153	3
526			min	-37.943	1	-41.259	2	1.863	12	0	3	-.044	1	-.327	2
527		17	max	-2.729	10	65.033	3	54.362	1	0	2	.012	5	.128	3
528			min	-37.943	1	-138.458	2	2.83	12	0	3	-.02	1	-.275	2
529		18	max	-2.729	10	110.002	3	79.133	1	0	2	.024	4	.077	3
530			min	-37.943	1	-235.657	2	3.797	12	0	3	0	10	-.166	2
531		19	max	-2.729	10	154.97	3	103.905	1	0	2	.073	1	0	2
532			min	-37.943	1	-332.856	2	4.765	12	0	3	.005	12	0	3
533	M15	1	max	0	1	1.202	9	.08	3	0	9	0	9	0	1
534			min	-76.083	3	0	1	-.023	9	0	3	0	3	0	1
535		2	max	0	1	1.069	9	.08	3	0	9	0	9	0	1
536			min	-76.153	3	0	1	-.023	9	0	3	0	3	0	9
537		3	max	0	1	.935	9	.08	3	0	9	0	9	0	1
538			min	-76.224	3	0	1	-.023	9	0	3	0	3	0	9
539		4	max	0	1	.801	9	.08	3	0	9	0	9	0	1
540			min	-76.294	3	0	1	-.023	9	0	3	0	3	-.001	9
541		5	max	0	1	.668	9	.08	3	0	9	0	9	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-76.365	3	0	1	-.023	9	0	3	0	3	-.001	9
543		6	max	0	1	.534	9	.08	3	0	9	0	9	0	1
544			min	-76.435	3	0	1	-.023	9	0	3	0	3	-.001	9
545		7	max	0	1	.401	9	.08	3	0	9	0	3	0	1
546			min	-76.506	3	0	1	-.023	9	0	3	0	9	-.002	9
547		8	max	0	1	.267	9	.08	3	0	9	0	3	0	1
548			min	-76.576	3	0	1	-.023	9	0	3	0	9	-.002	9
549		9	max	0	1	.134	9	.08	3	0	9	0	3	0	1
550			min	-76.647	3	0	1	-.023	9	0	3	0	9	-.002	9
551		10	max	0	1	0	1	.08	3	0	9	0	3	0	1
552			min	-76.717	3	0	1	-.023	9	0	3	0	9	-.002	9
553		11	max	0	1	0	1	.08	3	0	9	0	3	0	1
554			min	-76.788	3	-.134	9	-.023	9	0	3	0	9	-.002	9
555		12	max	0	1	0	1	.08	3	0	9	0	3	0	1
556			min	-76.858	3	-.267	9	-.023	9	0	3	0	9	-.002	9
557		13	max	0	1	0	1	.08	3	0	9	0	3	0	1
558			min	-76.929	3	-.401	9	-.023	9	0	3	0	9	-.002	9
559		14	max	0	1	0	1	.08	3	0	9	0	3	0	1
560			min	-76.999	3	-.534	9	-.023	9	0	3	0	9	-.001	9
561		15	max	0	1	0	1	.08	3	0	9	0	3	0	1
562			min	-77.07	3	-.668	9	-.023	9	0	3	0	9	-.001	9
563		16	max	0	1	0	1	.08	3	0	9	0	3	0	1
564			min	-77.14	3	-.801	9	-.023	9	0	3	0	9	-.001	9
565		17	max	0	1	0	1	.08	3	0	9	0	3	0	1
566			min	-77.211	3	-.935	9	-.023	9	0	3	0	9	0	9
567		18	max	0	1	0	1	.08	3	0	9	0	3	0	1
568			min	-77.281	3	-1.069	9	-.023	9	0	3	0	9	0	9
569		19	max	0	1	0	1	.08	3	0	9	0	3	0	1
570			min	-77.352	3	-1.202	9	-.023	9	0	3	0	9	0	1
571	M16A	1	max	0	10	2.572	4	.3	4	0	3	0	3	0	1
572			min	-198.407	4	0	10	-.033	3	0	2	0	4	0	1
573		2	max	0	10	2.286	4	.271	4	0	3	0	3	0	10
574			min	-198.423	4	0	10	-.033	3	0	2	0	4	0	4
575		3	max	0	10	2	4	.241	4	0	3	0	3	0	10
576			min	-198.439	4	0	10	-.033	3	0	2	0	4	-.002	4
577		4	max	0	10	1.714	4	.211	4	0	3	0	3	0	10
578			min	-198.455	4	0	10	-.033	3	0	2	0	4	-.002	4
579		5	max	0	10	1.429	4	.181	4	0	3	0	3	0	10
580			min	-198.471	4	0	10	-.033	3	0	2	0	1	-.003	4
581		6	max	0	10	1.143	4	.151	4	0	3	0	3	0	10
582			min	-198.487	4	0	10	-.033	3	0	2	0	1	-.003	4
583		7	max	0	10	.857	4	.121	4	0	3	0	5	0	10
584			min	-198.503	4	0	10	-.033	3	0	2	0	1	-.004	4
585		8	max	0	10	.571	4	.091	4	0	3	0	5	0	10
586			min	-198.519	4	0	10	-.033	3	0	2	0	1	-.004	4
587		9	max	0	10	.286	4	.061	4	0	3	0	5	0	10
588			min	-198.534	4	0	10	-.033	3	0	2	0	1	-.004	4
589		10	max	0	10	0	1	.031	4	0	3	0	5	0	10
590			min	-198.55	4	0	1	-.033	3	0	2	0	1	-.004	4
591		11	max	0	10	0	10	.03	1	0	3	0	5	0	10
592			min	-198.566	4	-.286	4	-.033	3	0	2	0	1	-.004	4
593		12	max	.089	2	0	10	.03	1	0	3	0	5	0	10
594			min	-198.582	4	-.571	4	-.033	3	0	2	0	1	-.004	4
595		13	max	.183	2	0	10	.03	1	0	3	0	5	0	10
596			min	-198.598	4	-.857	4	-.062	5	0	2	0	3	-.004	4
597		14	max	.277	2	0	10	.03	1	0	3	0	5	0	10
598			min	-198.614	4	-1.143	4	-.092	5	0	2	0	3	-.003	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.371	2	0	10	.03	1	0	3	0	4	0	10
600		min	-198.63	4	-1.429	4	-.122	5	0	2	0	3	-.003	4
601	16	max	.465	2	0	10	.03	1	0	3	0	4	0	10
602		min	-198.646	4	-1.714	4	-.151	5	0	2	0	3	-.002	4
603	17	max	.559	2	0	10	.03	1	0	3	0	2	0	10
604		min	-198.662	4	-2	4	-.181	5	0	2	0	3	-.002	4
605	18	max	.653	2	0	10	.03	1	0	3	0	2	0	10
606		min	-198.677	4	-2.286	4	-.211	5	0	2	0	5	0	4
607	19	max	.747	2	0	10	.03	1	0	3	0	2	0	1
608		min	-198.693	4	-2.572	4	-.241	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.009	2	.007	1	1.241e-3	5	NC	3	NC	2	
2			min	-.003	3	-.009	3	-.012	5	-5.833e-4	1	4374.501	2	5441.972	1	
3			2	max	.002	1	.008	2	.007	1	1.263e-3	5	NC	3	NC	2
4				min	-.003	3	-.009	3	-.012	5	-5.581e-4	1	4779.331	2	5862.31	1
5			3	max	.002	1	.007	2	.006	1	1.284e-3	5	NC	1	NC	2
6				min	-.003	3	-.008	3	-.012	5	-5.33e-4	1	5261.805	2	6359.016	1
7			4	max	.002	1	.007	2	.006	1	1.305e-3	5	NC	1	NC	2
8				min	-.003	3	-.008	3	-.011	5	-5.079e-4	1	5840.906	2	6950.346	1
9			5	max	.002	1	.006	2	.005	1	1.327e-3	5	NC	1	NC	2
10				min	-.003	3	-.008	3	-.011	5	-4.827e-4	1	6542.014	2	7660.691	1
11			6	max	.002	1	.005	2	.005	1	1.348e-3	5	NC	1	NC	2
12				min	-.002	3	-.007	3	-.011	5	-4.576e-4	1	7399.625	2	8523.206	1
13			7	max	.002	1	.005	2	.004	1	1.369e-3	5	NC	1	NC	2
14				min	-.002	3	-.007	3	-.01	5	-4.324e-4	1	8461.538	2	9583.901	1
15			8	max	.001	1	.004	2	.004	1	1.391e-3	5	NC	1	NC	1
16				min	-.002	3	-.006	3	-.01	5	-4.073e-4	1	9795.482	2	NC	1
17			9	max	.001	1	.003	2	.003	1	1.412e-3	5	NC	1	NC	1
18				min	-.002	3	-.006	3	-.009	5	-3.821e-4	1	NC	1	NC	1
19			10	max	.001	1	.003	2	.003	1	1.433e-3	5	NC	1	NC	1
20				min	-.002	3	-.005	3	-.008	5	-3.57e-4	1	NC	1	NC	1
21		11	max	.001	1	.002	2	.002	1	1.455e-3	5	NC	1	NC	1	
22			min	-.002	3	-.005	3	-.008	5	-3.318e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	1.476e-3	5	NC	1	NC	1	
24			min	-.001	3	-.004	3	-.007	5	-3.067e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	.001	1	1.497e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.006	5	-2.815e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.001	1	1.519e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.005	5	-2.564e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.54e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.004	5	-2.312e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.561e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.003	5	-2.061e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.583e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-1.809e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.604e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-1.558e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.625e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.306e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	6.163e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-7.661e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.004	5	7.601e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-7.737e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.008	5	9.038e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-7.814e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.012	5	1.048e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-7.89e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.016	5	1.191e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-7.966e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.02	4	1.335e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-8.042e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.023	4	1.479e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-8.118e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.027	4	1.622e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-8.195e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.031	4	1.766e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	-8.271e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.035	4	1.91e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-8.347e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.038	4	2.054e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-8.423e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.042	4	2.197e-4	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	10	-8.499e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.045	4	2.341e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-8.576e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.049	4	2.485e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	10	-8.652e-4	5	NC	1	NC	1
67		15	max	.001	3	.005	2	.052	4	2.629e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	-8.728e-4	5	9507.345	2	NC	1
69		16	max	.002	3	.006	2	.055	4	2.772e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-8.804e-4	5	8007.085	2	NC	1
71		17	max	.002	3	.007	2	.058	4	2.916e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-8.88e-4	5	6857.193	2	NC	1
73		18	max	.002	3	.008	2	.061	4	3.06e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-8.957e-4	5	5964.391	2	NC	1
75		19	max	.002	3	.009	2	.064	4	3.204e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-9.033e-4	5	5264.152	2	NC	1
77	M4	1	max	.002	1	.01	2	0	10	4.482e-3	5	NC	1	NC	2
78			min	0	12	-.009	3	-.068	4	-4.665e-4	1	NC	1	284.035	4
79		2	max	.001	1	.01	2	0	10	4.482e-3	5	NC	1	NC	2
80			min	0	12	-.008	3	-.062	4	-4.665e-4	1	NC	1	309.62	4
81		3	max	.001	1	.009	2	0	10	4.482e-3	5	NC	1	NC	2
82			min	0	12	-.008	3	-.057	4	-4.665e-4	1	NC	1	340.07	4
83		4	max	.001	1	.009	2	0	10	4.482e-3	5	NC	1	NC	2
84			min	0	12	-.007	3	-.051	4	-4.665e-4	1	NC	1	376.667	4
85		5	max	.001	1	.008	2	0	10	4.482e-3	5	NC	1	NC	1
86			min	0	12	-.007	3	-.046	4	-4.665e-4	1	NC	1	421.155	4
87		6	max	.001	1	.007	2	0	10	4.482e-3	5	NC	1	NC	1
88			min	0	12	-.006	3	-.041	4	-4.665e-4	1	NC	1	475.963	4
89		7	max	.001	1	.007	2	0	10	4.482e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.035	4	-4.665e-4	1	NC	1	544.548	4
91		8	max	0	1	.006	2	0	10	4.482e-3	5	NC	1	NC	1
92			min	0	12	-.005	3	-.031	4	-4.665e-4	1	NC	1	631.971	4
93		9	max	0	1	.006	2	0	10	4.482e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.026	4	-4.665e-4	1	NC	1	745.888	4
95		10	max	0	1	.005	2	0	10	4.482e-3	5	NC	1	NC	1
96			min	0	12	-.004	3	-.022	4	-4.665e-4	1	NC	1	898.334	4
97		11	max	0	1	.005	2	0	10	4.482e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.017	4	-4.665e-4	1	NC	1	1109.13	4
99		12	max	0	1	.004	2	0	10	4.482e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.003	3	-.014	4	-4.665e-4	1	NC	1	1412.847	4
101		max	0	1	.003	2	0	10	4.482e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.01	4	-4.665e-4	1	NC	1	1874.332	4
103		max	0	1	.003	2	0	10	4.482e-3	5	NC	1	NC	1
104		min	0	12	-.002	3	-.007	4	-4.665e-4	1	NC	1	2627.342	4
105		max	0	1	.002	2	0	10	4.482e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.005	4	-4.665e-4	1	NC	1	3986.107	4
107		max	0	1	.002	2	0	10	4.482e-3	5	NC	1	NC	1
108		min	0	12	-.001	3	-.003	4	-4.665e-4	1	NC	1	6843.894	4
109		max	0	1	.001	2	0	10	4.482e-3	5	NC	1	NC	1
110		min	0	12	0	3	-.001	4	-4.665e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	4.482e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-4.665e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	4.482e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-4.665e-4	1	NC	1	NC	1
115	M6	max	.007	1	.031	2	.003	1	1.339e-3	4	NC	3	NC	1
116		min	-.011	3	-.028	3	-.012	5	-6.264e-8	10	1271.928	2	7895.299	3
117		max	.007	1	.029	2	.003	1	1.359e-3	4	NC	3	NC	1
118		min	-.01	3	-.027	3	-.012	5	-9.945e-7	2	1361.211	2	8387.571	3
119		max	.006	1	.027	2	.002	1	1.379e-3	4	NC	3	NC	1
120		min	-.01	3	-.025	3	-.012	5	-2.106e-6	2	1463.546	2	8971.34	3
121		max	.006	1	.025	2	.002	1	1.399e-3	4	NC	3	NC	1
122		min	-.009	3	-.024	3	-.012	5	-4.196e-6	1	1581.547	2	9666.194	3
123		max	.006	1	.023	2	.002	1	1.419e-3	4	NC	3	NC	1
124		min	-.008	3	-.022	3	-.011	5	-7.78e-6	1	1718.58	2	NC	1
125		max	.005	1	.021	2	.002	1	1.439e-3	4	NC	3	NC	1
126		min	-.008	3	-.021	3	-.011	5	-1.136e-5	1	1879.056	2	NC	1
127		max	.005	1	.019	2	.002	1	1.459e-3	4	NC	3	NC	1
128		min	-.007	3	-.019	3	-.01	5	-1.495e-5	1	2068.858	2	NC	1
129		max	.004	1	.017	2	.001	1	1.479e-3	4	NC	3	NC	1
130		min	-.007	3	-.018	3	-.01	5	-1.853e-5	1	2296.012	2	NC	1
131		max	.004	1	.015	2	.001	1	1.499e-3	4	NC	3	NC	1
132		min	-.006	3	-.016	3	-.009	5	-2.212e-5	1	2571.755	2	NC	1
133		max	.004	1	.014	2	.001	1	1.519e-3	4	NC	3	NC	1
134		min	-.005	3	-.015	3	-.009	5	-2.57e-5	1	2912.32	2	NC	1
135		max	.003	1	.012	2	0	1	1.539e-3	4	NC	3	NC	1
136		min	-.005	3	-.013	3	-.008	5	-2.928e-5	1	3342.046	2	NC	1
137		max	.003	1	.01	2	0	1	1.56e-3	4	NC	3	NC	1
138		min	-.004	3	-.012	3	-.007	5	-3.287e-5	1	3899.175	2	NC	1
139		max	.002	1	.008	2	0	1	1.58e-3	4	NC	3	NC	1
140		min	-.004	3	-.01	3	-.006	5	-3.645e-5	1	4647.419	2	NC	1
141		max	.002	1	.007	2	0	1	1.6e-3	4	NC	3	NC	1
142		min	-.003	3	-.008	3	-.005	5	-4.004e-5	1	5701.433	2	NC	1
143		max	.002	1	.005	2	0	1	1.62e-3	4	NC	1	NC	1
144		min	-.002	3	-.007	3	-.004	5	-4.362e-5	1	7290.474	2	NC	1
145		max	.001	1	.004	2	0	1	1.64e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.003	5	-4.721e-5	1	9949.387	2	NC	1
147		max	0	1	.003	2	0	1	1.66e-3	4	NC	1	NC	1
148		min	-.001	3	-.003	3	-.002	5	-5.079e-5	1	NC	1	NC	1
149		max	0	1	.001	2	0	1	1.68e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-5.437e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.701e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.796e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.712e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-8.019e-4	5	NC	1	NC	1
155		max	0	3	.001	2	.004	5	2.31e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-7.968e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.008	5	1.909e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-7.922e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.012	5	1.507e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-7.877e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.016	5	1.106e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-7.831e-4	4	8574.094	2	NC	1
163		6	max	.002	3	.007	2	.02	5	2.257e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	-7.786e-4	4	6870.991	2	NC	1
165		7	max	.002	3	.008	2	.024	5	4.325e-5	3	NC	3	NC	1
166			min	-.002	2	-.011	3	0	1	-7.741e-4	4	5706.709	2	NC	1
167		8	max	.002	3	.009	2	.028	4	6.393e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-7.695e-4	4	4853.487	2	NC	1
169		9	max	.003	3	.011	2	.032	4	8.46e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	1	-7.65e-4	4	4197.835	2	NC	1
171		10	max	.003	3	.013	2	.036	4	1.053e-4	3	NC	3	NC	1
172			min	-.003	2	-.016	3	-.001	1	-7.604e-4	4	3676.838	2	NC	1
173		11	max	.003	3	.014	2	.039	4	1.26e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	-.001	1	-7.559e-4	4	3252.715	2	NC	1
175		12	max	.004	3	.016	2	.043	4	1.466e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	-.001	1	-7.513e-4	4	2901.285	2	NC	1
177		13	max	.004	3	.018	2	.046	4	1.673e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	-.001	1	-7.468e-4	4	2606.235	2	NC	1
179		14	max	.004	3	.02	2	.05	4	1.88e-4	3	NC	3	NC	1
180			min	-.005	2	-.021	3	-.001	1	-7.423e-4	4	2356.068	2	NC	1
181		15	max	.005	3	.022	2	.053	4	2.087e-4	3	NC	3	NC	1
182			min	-.005	2	-.022	3	-.001	1	-7.377e-4	4	2142.377	2	NC	1
183		16	max	.005	3	.024	2	.056	4	2.293e-4	3	NC	3	NC	1
184			min	-.006	2	-.023	3	-.001	1	-7.332e-4	4	1958.823	2	NC	1
185		17	max	.005	3	.026	2	.059	4	2.5e-4	3	NC	3	NC	1
186			min	-.006	2	-.024	3	-.001	1	-7.286e-4	4	1800.506	2	NC	1
187		18	max	.005	3	.028	2	.062	4	2.707e-4	3	NC	3	NC	1
188			min	-.006	2	-.024	3	-.002	1	-7.241e-4	4	1663.569	2	NC	1
189		19	max	.006	3	.03	2	.065	4	2.914e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	-.002	1	-7.196e-4	4	1544.928	2	NC	1
191	M8	1	max	.004	1	.035	2	.002	1	4.305e-3	4	NC	1	NC	1
192			min	0	3	-.028	3	-.069	4	-2.227e-4	3	NC	1	281.138	4
193		2	max	.004	1	.033	2	.001	1	4.305e-3	4	NC	1	NC	1
194			min	0	3	-.027	3	-.063	4	-2.227e-4	3	NC	1	306.462	4
195		3	max	.004	1	.031	2	.001	1	4.305e-3	4	NC	1	NC	1
196			min	0	3	-.025	3	-.057	4	-2.227e-4	3	NC	1	336.601	4
197		4	max	.004	1	.029	2	.001	1	4.305e-3	4	NC	1	NC	1
198			min	0	3	-.023	3	-.052	4	-2.227e-4	3	NC	1	372.825	4
199		5	max	.003	1	.027	2	.001	1	4.305e-3	4	NC	1	NC	1
200			min	0	3	-.022	3	-.046	4	-2.227e-4	3	NC	1	416.86	4
201		6	max	.003	1	.026	2	0	1	4.305e-3	4	NC	1	NC	1
202			min	0	3	-.02	3	-.041	4	-2.227e-4	3	NC	1	471.109	4
203		7	max	.003	1	.024	2	0	1	4.305e-3	4	NC	1	NC	1
204			min	0	3	-.019	3	-.036	4	-2.227e-4	3	NC	1	538.994	4
205		8	max	.003	1	.022	2	0	1	4.305e-3	4	NC	1	NC	1
206			min	0	3	-.017	3	-.031	4	-2.227e-4	3	NC	1	625.526	4
207		9	max	.002	1	.02	2	0	1	4.305e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.026	4	-2.227e-4	3	NC	1	738.283	4
209		10	max	.002	1	.018	2	0	1	4.305e-3	4	NC	1	NC	1
210			min	0	3	-.014	3	-.022	4	-2.227e-4	3	NC	1	889.176	4
211		11	max	.002	1	.016	2	0	1	4.305e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.018	4	-2.227e-4	3	NC	1	1097.824	4
213		12	max	.002	1	.014	2	0	1	4.305e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.007	4	4.152e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-7.605e-4	4	NC	1	NC	1
273	4	max	0	3	0	2	.01	4	2.072e-5	3	NC	1	NC	1
274		min	0	2	-.003	3	-.001	3	-8.241e-4	4	NC	1	NC	1
275	5	max	0	3	0	2	.013	4	-7.703e-8	3	NC	1	NC	1
276		min	0	2	-.003	3	-.001	3	-8.878e-4	4	NC	1	NC	1
277	6	max	0	3	0	2	.016	5	-9.689e-6	10	NC	1	NC	1
278		min	0	2	-.004	3	-.002	3	-9.514e-4	4	NC	1	NC	1
279	7	max	0	3	0	2	.02	5	-1.103e-5	10	NC	1	NC	1
280		min	0	2	-.005	3	-.002	3	-1.015e-3	4	NC	1	NC	1
281	8	max	0	3	0	2	.023	5	-1.236e-5	10	NC	1	NC	1
282		min	0	2	-.006	3	-.002	3	-1.079e-3	4	NC	1	NC	1
283	9	max	0	3	.001	2	.026	5	-1.37e-5	10	NC	1	NC	1
284		min	0	2	-.006	3	-.002	3	-1.142e-3	4	NC	1	NC	1
285	10	max	0	3	.002	2	.03	5	-1.504e-5	10	NC	1	NC	1
286		min	0	2	-.007	3	-.002	3	-1.206e-3	4	NC	1	NC	1
287	11	max	.001	3	.002	2	.033	5	-1.638e-5	10	NC	1	NC	1
288		min	-.001	2	-.007	3	-.003	1	-1.269e-3	4	NC	1	NC	1
289	12	max	.001	3	.003	2	.036	5	-1.771e-5	10	NC	1	NC	1
290		min	-.001	2	-.007	3	-.003	1	-1.333e-3	4	NC	1	NC	1
291	13	max	.001	3	.003	2	.039	5	-1.905e-5	10	NC	1	NC	1
292		min	-.001	2	-.008	3	-.004	1	-1.397e-3	4	NC	1	NC	1
293	14	max	.001	3	.004	2	.042	5	-2.039e-5	10	NC	1	NC	1
294		min	-.001	2	-.008	3	-.004	1	-1.46e-3	4	NC	1	NC	1
295	15	max	.002	3	.005	2	.045	5	-2.173e-5	10	NC	1	NC	2
296		min	-.001	2	-.008	3	-.005	1	-1.524e-3	4	9525.505	2	9653.768	1
297	16	max	.002	3	.006	2	.048	5	-2.307e-5	10	NC	1	NC	2
298		min	-.002	2	-.008	3	-.005	1	-1.588e-3	4	8020.847	2	8633.802	1
299	17	max	.002	3	.007	2	.051	5	-2.44e-5	10	NC	1	NC	2
300		min	-.002	2	-.008	3	-.006	1	-1.651e-3	4	6867.931	2	7817.154	1
301	18	max	.002	3	.008	2	.054	5	-2.574e-5	10	NC	1	NC	2
302		min	-.002	2	-.008	3	-.006	1	-1.715e-3	4	5973.003	2	7156.164	1
303	19	max	.002	3	.009	2	.057	5	-2.708e-5	10	NC	3	NC	2
304		min	-.002	2	-.008	3	-.007	1	-1.778e-3	4	5271.239	2	6617.05	1
305	M12	1	max	.002	1	.01	.006	1	5.258e-3	4	NC	1	NC	2
306		min	0	15	-.009	3	-.063	5	3.061e-5	10	NC	1	306.32	5
307	2	max	.001	1	.01	2	.005	1	5.258e-3	4	NC	1	NC	2
308		min	0	15	-.009	3	-.058	5	3.061e-5	10	NC	1	333.904	5
309	3	max	.001	1	.009	2	.005	1	5.258e-3	4	NC	1	NC	2
310		min	0	15	-.008	3	-.053	5	3.061e-5	10	NC	1	366.734	5
311	4	max	.001	1	.009	2	.004	1	5.258e-3	4	NC	1	NC	2
312		min	0	15	-.008	3	-.048	5	3.061e-5	10	NC	1	406.189	5
313	5	max	.001	1	.008	2	.004	1	5.258e-3	4	NC	1	NC	2
314		min	0	15	-.007	3	-.043	5	3.061e-5	10	NC	1	454.153	5
315	6	max	.001	1	.007	2	.003	1	5.258e-3	4	NC	1	NC	2
316		min	0	15	-.007	3	-.038	5	3.061e-5	10	NC	1	513.241	5
317	7	max	.001	1	.007	2	.003	1	5.258e-3	4	NC	1	NC	2
318		min	0	15	-.006	3	-.033	5	3.061e-5	10	NC	1	587.18	5
319	8	max	0	1	.006	2	.003	1	5.258e-3	4	NC	1	NC	2
320		min	0	15	-.006	3	-.028	5	3.061e-5	10	NC	1	681.426	5
321	9	max	0	1	.006	2	.002	1	5.258e-3	4	NC	1	NC	2
322		min	0	15	-.005	3	-.024	5	3.061e-5	10	NC	1	804.233	5
323	10	max	0	1	.005	2	.002	1	5.258e-3	4	NC	1	NC	1
324		min	0	15	-.005	3	-.02	5	3.061e-5	10	NC	1	968.573	5
325	11	max	0	1	.005	2	.001	1	5.258e-3	4	NC	1	NC	1
326		min	0	15	-.004	3	-.016	5	3.061e-5	10	NC	1	1195.812	5
327	12	max	0	1	.004	2	.001	1	5.258e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.013	5	3.061e-5	10	NC	1	1523.214	5
329		max	0	1	.003	2	0	1	5.258e-3	4	NC	1	NC	1
330		min	0	15	-.003	3	-.01	5	3.061e-5	10	NC	1	2020.68	5
331		max	0	1	.003	2	0	1	5.258e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.007	5	3.061e-5	10	NC	1	2832.386	5
333		max	0	1	.002	2	0	1	5.258e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.004	5	3.061e-5	10	NC	1	4297.038	5
335		max	0	1	.002	2	0	1	5.258e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.003	5	3.061e-5	10	NC	1	7377.469	5
337		max	0	1	.001	2	0	1	5.258e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.001	5	3.061e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	5.258e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	3.061e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	5.258e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	3.061e-5	10	NC	1	NC	1
343	M1	max	.008	3	.024	3	.007	5	9.345e-3	2	NC	1	NC	1
344		min	-.008	2	-.021	2	-.003	1	-1.312e-2	3	NC	1	NC	1
345		max	.008	3	.014	3	.009	5	4.582e-3	2	NC	4	NC	1
346		min	-.008	2	-.012	2	-.006	1	-6.483e-3	3	4897.869	3	NC	1
347		max	.008	3	.005	3	.012	5	4.027e-4	5	NC	4	NC	1
348		min	-.008	2	-.004	2	-.007	1	-3.541e-4	1	2537.459	3	8528.945	5
349		max	.008	3	.004	2	.016	5	4.093e-4	5	NC	4	NC	2
350		min	-.008	2	-.003	3	-.008	1	-3.035e-4	1	1808.585	3	5365.112	5
351		max	.008	3	.01	2	.019	5	4.159e-4	5	NC	4	NC	2
352		min	-.008	2	-.009	3	-.008	1	-2.528e-4	1	1451.551	2	3828.63	5
353		max	.008	3	.015	2	.023	5	4.225e-4	5	NC	4	NC	2
354		min	-.008	2	-.014	3	-.008	1	-2.022e-4	1	1232.757	2	2934.865	5
355		max	.008	3	.02	2	.027	5	4.291e-4	5	NC	4	NC	1
356		min	-.008	2	-.018	3	-.007	1	-1.515e-4	1	1097.694	2	2357.938	5
357		max	.008	3	.023	2	.031	5	4.357e-4	5	NC	4	NC	1
358		min	-.008	2	-.021	3	-.006	1	-1.009e-4	1	1012.833	2	1959.225	5
359		max	.008	3	.025	2	.035	5	4.423e-4	5	NC	4	NC	1
360		min	-.008	2	-.022	3	-.004	1	-5.021e-5	1	962.332	2	1669.78	4
361		max	.008	3	.026	2	.039	5	4.507e-4	4	NC	4	NC	1
362		min	-.008	2	-.023	3	-.002	1	-4.715e-6	9	938.618	2	1433.633	4
363		max	.008	3	.025	2	.043	4	4.691e-4	4	NC	4	NC	1
364		min	-.008	2	-.022	3	0	1	9.223e-6	10	938.933	2	1255.399	4
365		max	.008	3	.024	2	.048	4	4.875e-4	4	NC	4	NC	1
366		min	-.008	2	-.02	3	0	10	1.251e-5	10	964.315	2	1117.876	4
367		max	.008	3	.021	2	.053	4	5.06e-4	4	NC	4	NC	1
368		min	-.008	2	-.017	3	0	10	1.58e-5	10	1020.175	2	1009.99	4
369		max	.008	3	.016	2	.057	4	5.244e-4	4	NC	4	NC	2
370		min	-.008	2	-.013	3	0	10	1.909e-5	10	1119.059	2	924.352	4
371		max	.008	3	.01	2	.061	4	5.428e-4	4	NC	4	NC	2
372		min	-.008	2	-.008	3	0	10	2.238e-5	10	1288.552	2	855.888	4
373		max	.008	3	.003	2	.065	4	7.953e-4	4	NC	4	NC	2
374		min	-.008	2	-.003	3	0	10	2.48e-5	10	1596.249	2	801.035	4
375		max	.008	3	.004	3	.068	4	6.618e-3	4	NC	4	NC	1
376		min	-.008	2	-.006	2	0	10	-4.088e-5	1	2259.343	2	757.294	4
377		max	.008	3	.012	3	.071	4	6.655e-3	2	NC	4	NC	1
378		min	-.008	2	-.016	2	0	10	-3.21e-3	3	4377.435	2	722.748	4
379		max	.008	3	.02	3	.073	4	1.342e-2	2	NC	1	NC	1
380		min	-.008	2	-.028	2	-.002	1	-6.527e-3	3	NC	1	696.878	4
381	M5	max	.025	3	.077	3	.007	5	1.224e-5	4	NC	1	NC	1
382		min	-.029	2	-.069	2	-.003	1	0	2	NC	1	NC	1
383		max	.025	3	.046	3	.009	5	2.016e-4	5	NC	4	NC	1
384		min	-.029	2	-.04	2	-.003	1	-4.975e-5	1	1521.403	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.025	3	.016	3	.012	5	3.879e-4	5	NC	5	NC	1
386		min	-.029	2	-.013	2	-.003	1	-9.865e-5	1	788.479	3	NC	1
387	4	max	.025	3	.011	2	.016	5	4.047e-4	5	NC	5	NC	1
388		min	-.029	2	-.009	3	-.003	1	-9.4e-5	1	557.036	2	NC	1
389	5	max	.025	3	.032	2	.02	5	4.215e-4	5	NC	5	NC	1
390		min	-.029	2	-.029	3	-.003	1	-8.934e-5	1	440.466	2	NC	1
391	6	max	.025	3	.05	2	.024	5	4.382e-4	5	NC	5	NC	1
392		min	-.029	2	-.046	3	-.003	1	-8.469e-5	1	373.805	2	NC	1
393	7	max	.025	3	.064	2	.028	5	4.55e-4	5	NC	5	NC	1
394		min	-.029	2	-.058	3	-.003	1	-8.004e-5	1	332.631	2	9831.097	3
395	8	max	.025	3	.075	2	.032	5	4.717e-4	5	NC	5	NC	1
396		min	-.029	2	-.067	3	-.002	1	-7.538e-5	1	306.734	2	9689.752	3
397	9	max	.025	3	.082	2	.037	5	4.885e-4	5	NC	5	NC	1
398		min	-.029	2	-.071	3	-.002	1	-7.073e-5	1	291.288	2	9868.283	3
399	10	max	.025	3	.085	2	.041	5	5.053e-4	5	NC	5	NC	1
400		min	-.029	2	-.073	3	-.002	1	-6.607e-5	1	283.982	2	NC	1
401	11	max	.025	3	.084	2	.046	5	5.22e-4	5	NC	5	NC	1
402		min	-.029	2	-.07	3	-.002	1	-6.142e-5	1	283.97	2	NC	1
403	12	max	.025	3	.078	2	.05	5	5.388e-4	5	NC	5	NC	1
404		min	-.029	2	-.064	3	-.002	1	-5.676e-5	1	291.561	2	NC	1
405	13	max	.025	3	.068	2	.054	5	5.555e-4	5	NC	5	NC	1
406		min	-.029	2	-.055	3	-.002	1	-5.211e-5	1	308.389	2	NC	1
407	14	max	.024	3	.053	2	.058	4	5.723e-4	5	NC	5	NC	1
408		min	-.029	2	-.043	3	-.002	1	-4.745e-5	1	338.252	2	NC	1
409	15	max	.024	3	.034	2	.062	4	5.891e-4	5	NC	5	NC	1
410		min	-.029	2	-.027	3	-.002	1	-4.28e-5	1	389.515	2	NC	1
411	16	max	.024	3	.01	2	.066	4	8.371e-4	5	NC	5	NC	1
412		min	-.029	2	-.008	3	-.002	1	-4.145e-5	1	482.713	2	NC	1
413	17	max	.024	3	.014	3	.069	4	6.612e-3	4	NC	5	NC	1
414		min	-.029	2	-.02	2	-.002	1	-1.187e-4	1	684.175	2	NC	1
415	18	max	.024	3	.038	3	.071	4	3.392e-3	4	NC	4	NC	1
416		min	-.029	2	-.055	2	-.001	1	-6.067e-5	1	1326.479	2	NC	1
417	19	max	.024	3	.063	3	.073	4	3.686e-6	5	NC	1	NC	1
418		min	-.029	2	-.091	2	-.001	1	-6.541e-7	3	NC	1	NC	1
419	M9	1	max	.008	.024	.006	.006	5	1.312e-2	3	NC	1	NC	1
420		min	-.008	2	-.021	2	-.003	1	-9.345e-3	2	NC	1	NC	1
421	2	max	.008	3	.014	.005	.005	5	6.475e-3	3	NC	4	NC	1
422		min	-.008	2	-.012	2	0	9	-4.593e-3	2	4899.21	3	NC	1
423	3	max	.008	3	.005	.006	.006	4	1.428e-4	1	NC	4	NC	1
424		min	-.008	2	-.004	2	0	3	-5.156e-5	3	2538.164	3	NC	1
425	4	max	.008	3	.003	.007	.007	4	1.007e-4	1	NC	4	NC	1
426		min	-.008	2	-.003	3	-.001	3	-5.656e-5	3	1809.065	3	NC	1
427	5	max	.008	3	.01	.009	.009	4	5.854e-5	1	NC	4	NC	1
428		min	-.008	2	-.009	3	-.002	3	-6.155e-5	3	1452.583	2	NC	1
429	6	max	.008	3	.015	.011	.011	4	2.501e-5	2	NC	4	NC	1
430		min	-.008	2	-.014	3	-.003	3	-6.654e-5	3	1233.626	2	7753.279	4
431	7	max	.008	3	.02	.014	.014	4	9.773e-6	2	NC	4	NC	1
432		min	-.008	2	-.018	3	-.003	3	-7.154e-5	3	1098.458	2	5204.949	4
433	8	max	.008	3	.023	.017	.017	4	1.971e-5	5	NC	4	NC	1
434		min	-.008	2	-.021	3	-.004	3	-7.653e-5	3	1013.528	2	3763.709	4
435	9	max	.008	3	.025	.021	.021	5	3.169e-5	5	NC	5	NC	1
436		min	-.008	2	-.023	3	-.004	3	-1.1e-4	1	962.981	2	2868.714	4
437	10	max	.008	3	.026	.026	.026	5	4.367e-5	5	NC	5	NC	1
438		min	-.008	2	-.023	3	-.004	3	-1.521e-4	1	939.239	2	2274.175	4
439	11	max	.008	3	.025	.031	.031	5	5.564e-5	5	NC	5	NC	1
440		min	-.008	2	-.022	3	-.004	3	-1.942e-4	1	939.543	2	1858.653	4
441	12	max	.008	3	.024	.036	.036	5	6.762e-5	5	NC	4	NC	1



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 30° 130mph 30psf 5.25ft 7-10Pa Page 40



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.001	1	.101	3	.029	1	6.344e-3	2	NC	5	NC	2
500		min	-.073	4	-.194	2	-.003	10	-4.434e-3	3	758.429	2	3815.125	1
501	4	max	.001	1	.126	3	.043	1	7.418e-3	2	NC	5	NC	3
502		min	-.073	4	-.243	2	-.003	10	-5.155e-3	3	584.975	2	2640.963	1
503	5	max	.001	1	.136	3	.049	1	8.492e-3	2	NC	5	NC	10
504		min	-.073	4	-.261	2	-.004	10	-5.876e-3	3	539.476	2	2347.253	1
505	6	max	.001	1	.132	3	.044	1	9.566e-3	2	NC	5	NC	2
506		min	-.073	4	-.249	2	-.006	10	-6.598e-3	3	570.024	2	2573.814	1
507	7	max	.001	1	.116	3	.03	1	1.064e-2	2	NC	5	NC	2
508		min	-.073	4	-.211	2	-.009	10	-7.319e-3	3	685.816	2	3656.036	1
509	8	max	.001	1	.094	3	.026	3	1.171e-2	2	NC	5	NC	2
510		min	-.073	4	-.16	2	-.015	2	-8.04e-3	3	947.793	2	6757.46	3
511	9	max	.001	1	.072	3	.026	3	1.279e-2	2	NC	4	NC	1
512		min	-.073	4	-.113	2	-.025	2	-8.761e-3	3	1473.604	2	7047.264	3
513	10	max	.001	1	.063	3	.024	3	1.386e-2	2	NC	4	NC	4
514		min	-.073	4	-.091	2	-.029	2	-9.483e-3	3	1978.372	2	6184.787	2
515	11	max	.001	1	.072	3	.023	3	1.279e-2	2	NC	4	NC	1
516		min	-.073	4	-.113	2	-.025	2	-8.76e-3	3	1473.604	2	7770.614	2
517	12	max	.001	1	.094	3	.022	3	1.172e-2	2	NC	5	NC	2
518		min	-.073	4	-.16	2	-.015	2	-8.037e-3	3	947.793	2	8716.968	9
519	13	max	.001	1	.116	3	.03	1	1.064e-2	2	NC	5	NC	2
520		min	-.073	4	-.211	2	-.009	10	-7.315e-3	3	685.816	2	3650.115	1
521	14	max	.002	1	.132	3	.044	1	9.567e-3	2	NC	5	NC	2
522		min	-.073	4	-.249	2	-.006	10	-6.592e-3	3	570.024	2	2577.583	1
523	15	max	.002	1	.136	3	.049	1	8.493e-3	2	NC	5	NC	3
524		min	-.073	4	-.261	2	-.004	10	-5.87e-3	3	539.476	2	2356.263	1
525	16	max	.002	1	.126	3	.043	1	7.419e-3	2	NC	5	NC	3
526		min	-.073	4	-.243	2	-.004	5	-5.147e-3	3	584.975	2	2657.96	1
527	17	max	.002	1	.101	3	.028	1	6.346e-3	2	NC	5	NC	2
528		min	-.073	4	-.194	2	-.006	5	-4.424e-3	3	758.43	2	3853.733	1
529	18	max	.002	1	.064	3	.01	1	5.272e-3	2	NC	4	NC	2
530		min	-.073	4	-.118	2	-.005	5	-3.702e-3	3	1387.222	2	8823.856	1
531	19	max	.002	1	.02	3	.008	3	4.198e-3	2	NC	1	NC	1
532		min	-.073	4	-.028	2	-.008	2	-2.979e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.825e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.317e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.006	4	8.366e-4	3	NC	1	NC	1
536		min	0	5	-.005	1	0	3	-6.5e-4	5	NC	1	NC	1
537	3	max	0	3	0	5	.014	4	1.291e-3	3	NC	3	NC	1
538		min	-.001	5	-.009	1	-.003	3	-9.76e-4	2	7883.58	2	5320.36	4
539	4	max	0	3	.001	5	.022	4	1.745e-3	3	NC	5	NC	9
540		min	-.002	5	-.014	1	-.007	3	-1.437e-3	2	5408.596	2	3393.828	4
541	5	max	0	3	.002	5	.029	4	2.199e-3	3	NC	5	NC	9
542		min	-.002	5	-.018	1	-.012	3	-1.897e-3	2	4220.382	2	2524.882	4
543	6	max	0	3	.002	5	.036	4	2.653e-3	3	NC	5	9153.111	9
544		min	-.003	5	-.021	1	-.017	3	-2.358e-3	2	3551.898	2	2065.725	4
545	7	max	0	3	.003	5	.041	4	3.107e-3	3	NC	5	7257.503	9
546		min	-.004	5	-.024	1	-.022	3	-2.819e-3	2	3149.893	2	1810.07	4
547	8	max	0	3	.003	5	.044	4	3.561e-3	3	NC	5	6049.899	9
548		min	-.004	5	-.025	1	-.027	3	-3.279e-3	2	2908.629	2	1675.929	4
549	9	max	0	3	.004	5	.046	4	4.016e-3	3	NC	5	5253.048	9
550		min	-.005	5	-.027	1	-.032	3	-3.74e-3	2	2778.767	2	1628.532	4
551	10	max	0	3	.004	5	.045	4	4.47e-3	3	NC	5	4725.634	9
552		min	-.005	5	-.027	1	-.036	3	-4.201e-3	2	2737.684	2	1457.957	3
553	11	max	0	3	.004	5	.042	4	4.924e-3	3	NC	5	4393.035	9
554		min	-.006	5	-.027	1	-.038	3	-4.661e-3	2	2778.767	2	1348.307	3
555	12	max	0	3	.005	5	.038	4	5.378e-3	3	NC	5	4218.844	9



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.006	5	-0.025	1	-0.039	3	-5.122e-3	2	2908.629	2	1289.164	3
557	13	max	.001	3	.005	5	.033	1	5.832e-3	3	NC	5	4353.146	15
558		min	-0.007	5	-0.023	1	-0.038	3	-5.583e-3	2	3149.893	2	1276.895	3
559	14	max	.001	3	.005	5	.03	1	6.286e-3	3	NC	5	5934.928	15
560		min	-0.008	5	-0.021	1	-0.034	3	-6.043e-3	2	3551.898	2	1317.157	3
561	15	max	.001	3	.005	5	.025	1	6.74e-3	3	NC	5	9432.049	15
562		min	-0.008	5	-0.017	1	-0.028	3	-6.504e-3	2	4220.382	2	1430.486	3
563	16	max	.001	3	.005	5	.018	1	7.195e-3	3	NC	5	NC	5
564		min	-0.009	5	-0.014	1	-0.019	3	-6.965e-3	2	5408.596	2	1672.559	3
565	17	max	.001	3	.006	5	.007	1	7.649e-3	3	NC	3	NC	4
566		min	-0.009	5	-0.01	9	-0.006	3	-7.425e-3	2	7883.58	2	2217.971	3
567	18	max	.001	3	.006	5	.01	3	8.103e-3	3	NC	1	NC	4
568		min	-.01	5	-0.006	9	-0.013	2	-7.886e-3	2	NC	1	3949.863	3
569	19	max	.002	3	.006	5	.031	3	8.557e-3	3	NC	1	NC	1
570		min	-0.011	5	-0.002	9	-0.031	2	-8.347e-3	2	NC	1	NC	1
571	M16A	1	max	0	0	10	.009	3	2.477e-3	3	NC	1	NC	1
572		min	-0.004	4	-0.004	4	-0.009	2	-2.427e-3	2	NC	1	NC	1
573	2	max	0	10	-0.003	12	.002	3	2.381e-3	3	NC	1	NC	1
574		min	-0.004	4	-0.012	4	-0.003	2	-2.319e-3	2	8690.523	4	NC	1
575	3	max	0	10	-0.005	12	.006	1	2.284e-3	3	NC	3	NC	4
576		min	-0.003	4	-0.02	4	-0.008	5	-2.212e-3	2	4422.31	4	6352.551	3
577	4	max	0	10	-0.007	12	.01	1	2.187e-3	3	NC	12	NC	9
578		min	-0.003	4	-0.028	4	-0.014	5	-2.104e-3	2	3033.963	4	4838.182	3
579	5	max	0	10	-0.009	12	.013	1	2.091e-3	3	8440.765	12	NC	10
580		min	-0.003	4	-0.034	4	-0.022	5	-1.996e-3	2	2367.432	4	3661.879	5
581	6	max	0	10	-0.011	12	.014	1	1.994e-3	3	7103.796	12	9737.371	14
582		min	-0.003	4	-0.04	4	-0.029	5	-1.888e-3	2	1992.445	4	2639.494	5
583	7	max	0	10	-0.012	12	.015	1	1.897e-3	3	6299.785	12	9260.226	10
584		min	-0.003	4	-0.045	4	-0.037	5	-1.78e-3	2	1766.939	4	2094.472	5
585	8	max	0	10	-0.013	12	.015	1	1.801e-3	3	5817.258	12	9560.47	10
586		min	-0.002	4	-0.048	4	-0.043	5	-1.672e-3	2	1631.602	4	1782.886	5
587	9	max	0	10	-0.014	12	.014	1	1.704e-3	3	5557.533	12	NC	10
588		min	-0.002	4	-0.05	4	-0.047	5	-1.564e-3	2	1558.755	4	1605.079	5
589	10	max	0	10	-0.014	12	.013	1	1.607e-3	3	5475.369	12	NC	10
590		min	-0.002	4	-0.05	4	-0.05	5	-1.457e-3	2	1535.71	4	1516.342	5
591	11	max	0	10	-0.014	12	.011	1	1.511e-3	3	5557.533	12	NC	9
592		min	-0.002	4	-0.049	4	-0.05	5	-1.349e-3	2	1558.755	4	1497.689	5
593	12	max	0	10	-0.013	12	.009	1	1.414e-3	3	5817.258	12	NC	9
594		min	-0.002	4	-0.047	4	-0.049	5	-1.241e-3	2	1631.602	4	1545.872	5
595	13	max	0	10	-0.012	12	.007	1	1.318e-3	3	6299.785	12	NC	2
596		min	-0.001	4	-0.043	4	-0.045	5	-1.133e-3	2	1766.939	4	1672.06	5
597	14	max	0	10	-0.011	12	.005	1	1.221e-3	3	7103.796	12	NC	1
598		min	-0.001	4	-0.038	4	-0.039	5	-1.025e-3	2	1992.445	4	1908.222	5
599	15	max	0	10	-0.009	12	.003	1	1.124e-3	3	8440.765	12	NC	1
600		min	0	4	-0.032	4	-0.032	5	-9.172e-4	2	2367.432	4	2328.617	5
601	16	max	0	10	-0.007	12	.002	1	1.028e-3	3	NC	12	NC	1
602		min	0	4	-0.025	4	-0.024	5	-8.094e-4	2	3033.963	4	3118.772	5
603	17	max	0	10	-0.005	12	0	9	9.309e-4	3	NC	3	NC	1
604		min	0	4	-0.017	4	-0.015	5	-7.015e-4	2	4422.31	4	4858.126	5
605	18	max	0	10	-0.002	12	0	3	9.617e-4	4	NC	1	NC	1
606		min	0	4	-0.009	4	-0.007	5	-5.936e-4	2	8690.523	4	NC	1
607	19	max	0	1	0	1	0	1	1.029e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.858e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.