

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	25° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	75 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.577 k-ft
$M_z$ =	-0.008 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>57%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.210 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.718 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>97%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.725 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>10%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

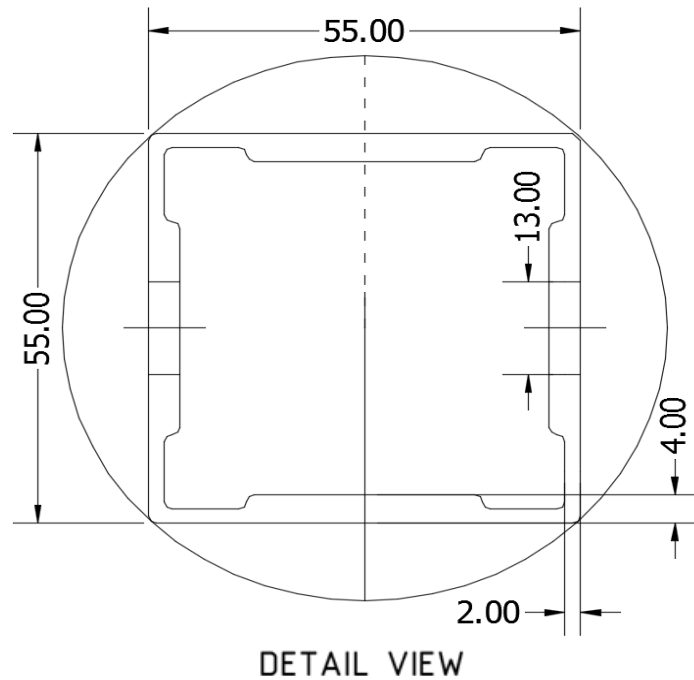
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.366 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>40%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	69.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.82 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.223 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	10.629 k
Utilization =	<b>31%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

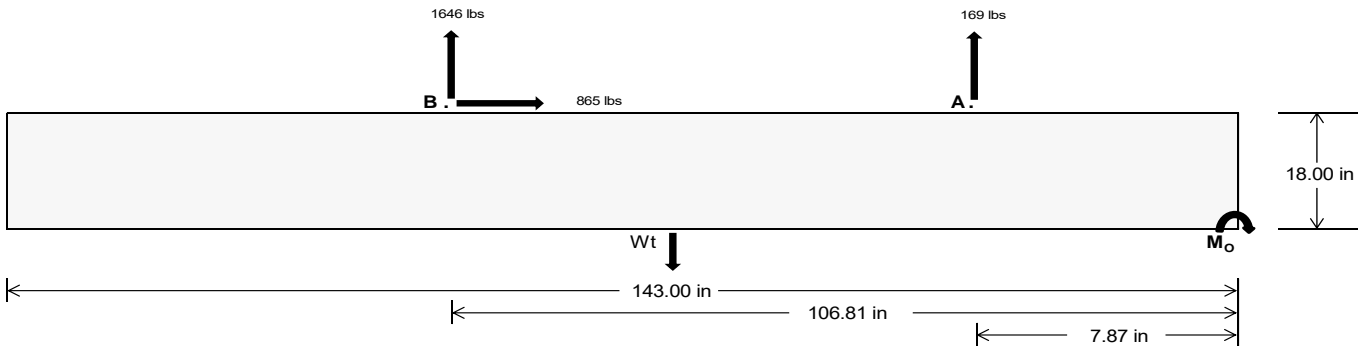
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<b>712.08</b>	<b>6854.05</b> k
Compressive Load =		<b>3542.83</b>	<b>4939.47</b> k
Lateral Load =		<b>7.96</b>	<b>3595.48</b> k
Moment (Weak Axis) =		<b>0.02</b>	<b>0.00</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 192730.6$  in-lbs  
Resisting Force Required = 2695.53 lbs  
S.F. = 1.67  
Weight Required = 4492.55 lbs  
Minimum Width = 36 in  
Weight Provided = 7775.63 lbs

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 864.67 lbs  
Friction = 0.4  
Weight Required = 2161.68 lbs  
Resisting Weight = 7775.63 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 36in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 864.67 lbs  
Cohesion = 130 psf  
Area = 35.75 ft<sup>2</sup>  
Resisting = 3887.81 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 36in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(3 \text{ ft}) =$   
36 in 37 in 38 in 39 in  
7776 lbs 7992 lbs 8208 lbs 8424 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in
$F_A$	983 lbs	983 lbs	983 lbs	983 lbs	1501 lbs	1501 lbs	1501 lbs	1501 lbs	1769 lbs	1769 lbs	1769 lbs	1769 lbs	-338 lbs	-338 lbs	-338 lbs	-338 lbs
$F_B$	996 lbs	996 lbs	996 lbs	996 lbs	2168 lbs	2168 lbs	2168 lbs	2168 lbs	2279 lbs	2279 lbs	2279 lbs	2279 lbs	-3292 lbs	-3292 lbs	-3292 lbs	-3292 lbs
$F_V$	97 lbs	97 lbs	97 lbs	97 lbs	1533 lbs	1533 lbs	1533 lbs	1533 lbs	1215 lbs	1215 lbs	1215 lbs	1215 lbs	-1729 lbs	-1729 lbs	-1729 lbs	-1729 lbs
$P_{total}$	9755 lbs	9971 lbs	10187 lbs	10403 lbs	11445 lbs	11661 lbs	11877 lbs	12093 lbs	11824 lbs	12040 lbs	12256 lbs	12472 lbs	1035 lbs	1164 lbs	1294 lbs	1424 lbs
$M$	2424 lbs-ft	2424 lbs-ft	2424 lbs-ft	2424 lbs-ft	3880 lbs-ft	3880 lbs-ft	3880 lbs-ft	3880 lbs-ft	4498 lbs-ft	4498 lbs-ft	4498 lbs-ft	4498 lbs-ft	5302 lbs-ft	5302 lbs-ft	5302 lbs-ft	5302 lbs-ft
$e$	0.25 ft	0.24 ft	0.24 ft	0.23 ft	0.34 ft	0.33 ft	0.33 ft	0.32 ft	0.38 ft	0.37 ft	0.37 ft	0.37 ft	0.36 ft	5.12 ft	4.55 ft	4.10 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	238.7 psf	238.1 psf	237.6 psf	237.1 psf	265.5 psf	264.2 psf	263.0 psf	261.8 psf	267.4 psf	266.1 psf	264.8 psf	263.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	307.0 psf	304.6 psf	302.3 psf	300.1 psf	374.8 psf	370.5 psf	366.5 psf	362.7 psf	394.1 psf	389.3 psf	384.8 psf	380.5 psf	275.4 psf	179.2 psf	146.4 psf	130.7 psf

Maximum Bearing Pressure = 394 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

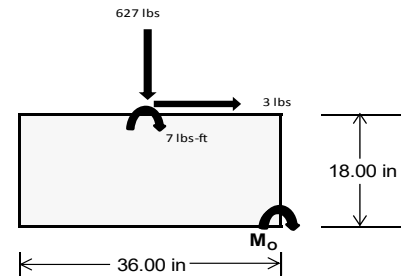
#### Overturning Check

$M_o = 928.3 \text{ ft-lbs}$   
 Resisting Force Required = 618.87 lbs  
 S.F. = 1.67  
 Weight Required = 1031.45 lbs  
 Minimum Width = 36 in  
 Weight Provided = 7775.63 lbs

*A minimum 143in long x 36in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	36 in			36 in			36 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	203 lbs	465 lbs	203 lbs	627 lbs	1645 lbs	627 lbs	59 lbs	136 lbs	59 lbs
$F_v$	1 lbs	0 lbs	1 lbs	3 lbs	0 lbs	3 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9830 lbs	7776 lbs	9830 lbs	9790 lbs	7776 lbs	9790 lbs	2874 lbs	7776 lbs	2874 lbs
M	3 lbs-ft	0 lbs-ft	3 lbs-ft	12 lbs-ft	0 lbs-ft	12 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
L/6	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft
$f_{min}$	274.8 psf	217.5 psf	274.8 psf	273.2 psf	217.5 psf	273.2 psf	80.4 psf	217.5 psf	80.4 psf
$f_{max}$	275.1 psf	217.5 psf	275.1 psf	274.5 psf	217.5 psf	274.5 psf	80.4 psf	217.5 psf	80.4 psf



Maximum Bearing Pressure = 275 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	1.182 k
Allowable Uplift =	1.214 k
Utilization =	<u>97%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.647 k
Allowable Uplift =	4.357 k
Utilization =	<u>61%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.725 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>37%</u>

#### Rear Strut

Maximum Axial Load =	4.671 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>63%</u>

#### Diagonal Strut

Maximum Axial Load =	2.556 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>34%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	56.48 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.130 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 75 \text{ in}$$

$$J = 0.432$$

$$207.485$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 75$$

$$J = 0.432$$

$$131.948$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.80 \text{ in} \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 69.8 \\ J &= 0.942 \\ &= 108.93 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.0 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.61471$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.80606$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 10.8205 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 10.82 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 11.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-55.176	-55.176	0	0
2	M14	Y	-55.176	-55.176	0	0
3	M15	Y	-55.176	-55.176	0	0
4	M16	Y	-55.176	-55.176	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-95.761	-95.761	0	0
2	M14	y	-95.761	-95.761	0	0
3	M15	y	-147.995	-147.995	0	0
4	M16	y	-147.995	-147.995	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	217.64	217.64	0	0
2	M14	y	165.406	165.406	0	0
3	M15	y	87.056	87.056	0	0
4	M16	y	87.056	87.056	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes	Y		1	1.2	3	1.6	4	.8										
2	LRFD 1.2D + 1.6W + 0.5S	Yes	Y		1	1.2	3	.5	4	1.6										
3	LRFD 0.9D + 1.6W	Yes	Y		2	.9					5	1.6								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	36.124	1	227.393	2	3.75	3	.012	3	-.003	15	.776	3
28			min	1.535	15	-371.204	3	-25.122	1	0	15	-.072	1	-.359	2
29		15	max	36.124	1	95.058	2	5.588	3	.012	3	-.003	12	.951	3
30			min	1.535	15	-133.323	3	-1.609	10	0	15	-.079	1	-.471	2
31		16	max	36.124	1	104.558	3	34.689	1	.012	3	0	3	.961	3
32			min	1.535	15	-37.276	2	1.475	15	0	15	-.065	1	-.491	2
33		17	max	36.124	1	342.438	3	64.595	1	.012	3	.006	3	.806	3
34			min	1.535	15	-169.611	2	2.68	15	0	15	-.031	1	-.419	2
35		18	max	36.124	1	580.319	3	94.5	1	.012	3	.024	1	.486	3
36			min	1.535	15	-301.945	2	3.886	15	0	15	-.002	10	-.256	2
37		19	max	36.124	1	818.2	3	124.406	1	.012	3	.1	1	0	2
38			min	1.535	15	-434.28	2	5.091	15	0	15	.004	15	0	3
39	M14	1	max	26.599	1	541.811	2	-5.351	15	.016	3	.126	1	0	2
40			min	1.119	15	-682.267	3	-130.7	1	-.017	2	.005	15	0	3
41		2	max	26.599	1	409.477	2	-4.145	15	.016	3	.046	1	.411	3
42			min	1.119	15	-502.424	3	-100.794	1	-.017	2	0	10	-.33	2
43		3	max	26.599	1	277.142	2	-2.94	15	.016	3	.009	3	.698	3
44			min	1.119	15	-322.581	3	-70.889	1	-.017	2	-.014	1	-.569	2
45		4	max	26.599	1	144.808	2	-1.734	15	.016	3	.002	3	.859	3
46			min	1.119	15	-142.738	3	-40.983	1	-.017	2	-.053	1	-.715	2
47		5	max	26.599	1	37.105	3	1.078	10	.016	3	-.002	12	.896	3
48			min	1.119	15	-.685	9	-11.078	1	-.017	2	-.071	1	-.77	2
49		6	max	26.599	1	216.948	3	18.828	1	.016	3	-.003	15	.808	3
50			min	1.119	15	-119.861	2	-4.382	3	-.017	2	-.068	1	-.733	2
51		7	max	26.599	1	396.791	3	48.733	1	.016	3	-.002	15	.595	3
52			min	1.119	15	-252.196	2	-2.544	3	-.017	2	-.045	1	-.603	2
53		8	max	26.599	1	576.634	3	78.639	1	.016	3	.006	2	.257	3
54			min	1.119	15	-384.53	2	-.706	3	-.017	2	-.01	3	-.382	2
55		9	max	26.599	1	756.477	3	108.545	1	.016	3	.065	1	.004	9
56			min	1.119	15	-516.865	2	.973	12	-.017	2	-.01	3	-.206	3
57		10	max	26.599	1	936.32	3	138.45	1	.017	2	.15	1	.336	2
58			min	1.119	15	-649.199	2	-73.309	14	-.016	3	-.008	3	-.794	3
59		11	max	26.599	1	516.865	2	-.973	12	.017	2	.065	1	.004	9
60			min	1.119	15	-756.477	3	-108.545	1	-.016	3	-.01	3	-.206	3
61		12	max	26.599	1	384.53	2	.706	3	.017	2	.006	2	.257	3
62			min	1.119	15	-576.634	3	-78.639	1	-.016	3	-.01	3	-.382	2
63		13	max	26.599	1	252.196	2	2.544	3	.017	2	-.002	15	.595	3
64			min	1.119	15	-396.791	3	-48.733	1	-.016	3	-.045	1	-.603	2
65		14	max	26.599	1	119.861	2	4.382	3	.017	2	-.003	15	.808	3
66			min	1.119	15	-216.948	3	-18.828	1	-.016	3	-.068	1	-.733	2
67		15	max	26.599	1	.685	9	11.078	1	.017	2	-.002	12	.896	3
68			min	1.119	15	-37.105	3	-1.078	10	-.016	3	-.071	1	-.77	2
69		16	max	26.599	1	142.738	3	40.983	1	.017	2	.002	3	.859	3
70			min	1.119	15	-144.808	2	1.734	15	-.016	3	-.053	1	-.715	2
71		17	max	26.599	1	322.581	3	70.889	1	.017	2	.009	3	.698	3
72			min	1.119	15	-277.142	2	2.94	15	-.016	3	-.014	1	-.569	2
73		18	max	26.599	1	502.424	3	100.794	1	.017	2	.046	1	.411	3
74			min	1.119	15	-409.477	2	4.145	15	-.016	3	0	10	-.33	2
75		19	max	26.599	1	682.267	3	130.7	1	.017	2	.126	1	0	2
76			min	1.119	15	-541.811	2	5.351	15	-.016	3	.005	15	0	3
77	M15	1	max	-1.174	15	738.01	2	-5.348	15	.017	2	.126	1	0	2
78			min	-27.558	1	-390.041	3	-130.762	1	-.013	3	.005	15	0	3
79		2	max	-1.174	15	547.638	2	-4.143	15	.017	2	.046	1	.239	3
80			min	-27.558	1	-297.253	3	-100.856	1	-.013	3	0	10	-.446	2
81		3	max	-1.174	15	357.265	2	-2.937	15	.017	2	.008	3	.413	3
82			min	-27.558	1	-204.466	3	-70.951	1	-.013	3	-.014	1	-.761	2
83		4	max	-1.174	15	166.893	2	-1.731	15	.017	2	.002	3	.523	3



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Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-27.558	1	-111.678	3	-41.045	1	-.013	3	-.053	1	-.943	2
85		5	max	-1.174	15	-.049	15	.927	10	.017	2	-.002	12	.568	3
86			min	-27.558	1	-23.479	2	-11.139	1	-.013	3	-.071	1	-.992	2
87		6	max	-1.174	15	73.897	3	18.766	1	.017	2	-.003	15	.549	3
88			min	-27.558	1	-213.851	2	-3.908	3	-.013	3	-.068	1	-.91	2
89		7	max	-1.174	15	166.684	3	48.672	1	.017	2	-.002	15	.465	3
90			min	-27.558	1	-404.224	2	-2.071	3	-.013	3	-.045	1	-.695	2
91		8	max	-1.174	15	259.472	3	78.577	1	.017	2	.005	2	.317	3
92			min	-27.558	1	-594.596	2	-.233	3	-.013	3	-.009	3	-.349	2
93		9	max	-1.174	15	352.259	3	108.483	1	.017	2	.064	1	.13	2
94			min	-27.558	1	-784.968	2	1.27	12	-.013	3	-.009	3	0	15
95		10	max	-1.174	15	445.047	3	138.389	1	.013	3	.15	1	.742	2
96			min	-27.558	1	-975.34	2	-73.345	14	-.017	2	-.007	3	-.172	3
97		11	max	-1.174	15	784.968	2	-1.27	12	.013	3	.064	1	.13	2
98			min	-27.558	1	-352.259	3	-108.483	1	-.017	2	-.009	3	0	15
99		12	max	-1.174	15	594.596	2	.233	3	.013	3	.005	2	.317	3
100			min	-27.558	1	-259.472	3	-78.577	1	-.017	2	-.009	3	-.349	2
101		13	max	-1.174	15	404.224	2	2.071	3	.013	3	-.002	15	.465	3
102			min	-27.558	1	-166.684	3	-48.672	1	-.017	2	-.045	1	-.695	2
103		14	max	-1.174	15	213.851	2	3.908	3	.013	3	-.003	15	.549	3
104			min	-27.558	1	-73.897	3	-18.766	1	-.017	2	-.068	1	-.91	2
105		15	max	-1.174	15	23.479	2	11.139	1	.013	3	-.002	12	.568	3
106			min	-27.558	1	.049	15	-.927	10	-.017	2	-.071	1	-.992	2
107		16	max	-1.174	15	111.678	3	41.045	1	.013	3	.002	3	.523	3
108			min	-27.558	1	-166.893	2	1.731	15	-.017	2	-.053	1	-.943	2
109		17	max	-1.174	15	204.466	3	70.951	1	.013	3	.008	3	.413	3
110			min	-27.558	1	-357.265	2	2.937	15	-.017	2	-.014	1	-.761	2
111		18	max	-1.174	15	297.253	3	100.856	1	.013	3	.046	1	.239	3
112			min	-27.558	1	-547.638	2	4.143	15	-.017	2	0	10	-.446	2
113		19	max	-1.174	15	390.041	3	130.762	1	.013	3	.126	1	0	2
114			min	-27.558	1	-738.01	2	5.348	15	-.017	2	.005	15	0	3
115	M16	1	max	-1.726	15	637.118	2	-5.105	15	.003	1	.103	1	0	2
116			min	-40.917	1	-298.702	3	-125.142	1	-.011	3	.004	15	0	3
117		2	max	-1.726	15	446.746	2	-3.9	15	.003	1	.026	1	.175	3
118			min	-40.917	1	-205.915	3	-95.237	1	-.011	3	0	10	-.376	2
119		3	max	-1.726	15	256.374	2	-2.694	15	.003	1	.004	3	.286	3
120			min	-40.917	1	-113.127	3	-65.331	1	-.011	3	-.03	1	-.62	2
121		4	max	-1.726	15	66.001	2	-1.488	15	.003	1	0	12	.332	3
122			min	-40.917	1	-20.34	3	-35.425	1	-.011	3	-.065	1	-.732	2
123		5	max	-1.726	15	72.448	3	1.079	10	.003	1	-.003	12	.314	3
124			min	-40.917	1	-124.371	2	-5.52	1	-.011	3	-.079	1	-.712	2
125		6	max	-1.726	15	165.235	3	24.386	1	.003	1	-.003	15	.232	3
126			min	-40.917	1	-314.743	2	-2.284	3	-.011	3	-.072	1	-.56	2
127		7	max	-1.726	15	258.023	3	54.291	1	.003	1	-.002	15	.085	3
128			min	-40.917	1	-505.115	2	-.447	3	-.011	3	-.045	1	-.275	2
129		8	max	-1.726	15	350.81	3	84.197	1	.003	1	.006	2	.142	2
130			min	-40.917	1	-695.488	2	1.077	12	-.011	3	-.007	3	-.127	3
131		9	max	-1.726	15	443.598	3	114.102	1	.003	1	.072	1	.691	2
132			min	-40.917	1	-885.86	2	2.302	12	-.011	3	-.006	3	-.402	3
133		10	max	-1.726	15	536.385	3	144.008	1	.011	3	.162	1	1.372	2
134			min	-40.917	1	-1076.232	2	-76.586	14	-.003	1	-.003	3	-.743	3
135		11	max	-1.726	15	885.86	2	-2.302	12	.011	3	.072	1	.691	2
136			min	-40.917	1	-443.598	3	-114.102	1	-.003	1	-.006	3	-.402	3
137		12	max	-1.726	15	695.488	2	-1.077	12	.011	3	.006	2	.142	2
138			min	-40.917	1	-350.81	3	-84.197	1	-.003	1	-.007	3	-.127	3
139		13	max	-1.726	15	505.115	2	.447	3	.011	3	-.002	15	.085	3
140			min	-40.917	1	-258.023	3	-54.291	1	-.003	1	-.045	1	-.275	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-1.726	15	314.743	2	2.284	3	.011	3	-.003	15	.232	3
142		min	-40.917	1	-165.235	3	-24.386	1	-.003	1	-.072	1	-.56	2
143	15	max	-1.726	15	124.371	2	5.52	1	.011	3	-.003	12	.314	3
144		min	-40.917	1	-72.448	3	-1.079	10	-.003	1	-.079	1	-.712	2
145	16	max	-1.726	15	20.34	3	35.425	1	.011	3	0	12	.332	3
146		min	-40.917	1	-66.001	2	1.488	15	-.003	1	-.065	1	-.732	2
147	17	max	-1.726	15	113.127	3	65.331	1	.011	3	.004	3	.286	3
148		min	-40.917	1	-256.374	2	2.694	15	-.003	1	-.03	1	-.62	2
149	18	max	-1.726	15	205.915	3	95.237	1	.011	3	.026	1	.175	3
150		min	-40.917	1	-446.746	2	3.9	15	-.003	1	0	10	-.376	2
151	19	max	-1.726	15	298.702	3	125.142	1	.011	3	.103	1	0	2
152		min	-40.917	1	-637.118	2	5.105	15	-.003	1	.004	15	0	3
153	M2	1	max	1113.549	2	2.026	4	.284	1	0	3	0	3	1
154		min	-1562.185	3	.476	15	.012	15	0	1	0	2	0	1
155	2	max	1114.023	2	1.989	4	.284	1	0	3	0	1	0	15
156		min	-1561.83	3	.468	15	.012	15	0	1	0	10	0	4
157	3	max	1114.497	2	1.952	4	.284	1	0	3	0	1	0	15
158		min	-1561.475	3	.459	15	.012	15	0	1	0	10	-.001	4
159	4	max	1114.97	2	1.915	4	.284	1	0	3	0	1	0	15
160		min	-1561.119	3	.45	15	.012	15	0	1	0	10	-.002	4
161	5	max	1115.444	2	1.878	4	.284	1	0	3	0	1	0	15
162		min	-1560.764	3	.441	15	.012	15	0	1	0	10	-.002	4
163	6	max	1115.918	2	1.841	4	.284	1	0	3	0	1	0	15
164		min	-1560.409	3	.433	15	.012	15	0	1	0	15	-.003	4
165	7	max	1116.391	2	1.804	4	.284	1	0	3	0	1	0	15
166		min	-1560.053	3	.424	15	.012	15	0	1	0	15	-.004	4
167	8	max	1116.865	2	1.767	4	.284	1	0	3	0	1	0	15
168		min	-1559.698	3	.415	15	.012	15	0	1	0	15	-.004	4
169	9	max	1117.339	2	1.73	4	.284	1	0	3	0	1	-.001	15
170		min	-1559.343	3	.407	15	.012	15	0	1	0	15	-.005	4
171	10	max	1117.813	2	1.693	4	.284	1	0	3	0	1	-.001	15
172		min	-1558.987	3	.398	15	.012	15	0	1	0	15	-.005	4
173	11	max	1118.286	2	1.656	4	.284	1	0	3	0	1	-.001	15
174		min	-1558.632	3	.389	15	.012	15	0	1	0	15	-.006	4
175	12	max	1118.76	2	1.619	4	.284	1	0	3	0	1	-.002	15
176		min	-1558.277	3	.38	15	.012	15	0	1	0	15	-.006	4
177	13	max	1119.234	2	1.581	4	.284	1	0	3	.001	1	-.002	15
178		min	-1557.921	3	.372	15	.012	15	0	1	0	15	-.007	4
179	14	max	1119.708	2	1.544	4	.284	1	0	3	.001	1	-.002	15
180		min	-1557.566	3	.363	15	.012	15	0	1	0	15	-.007	4
181	15	max	1120.181	2	1.507	4	.284	1	0	3	.001	1	-.002	15
182		min	-1557.211	3	.354	12	.012	15	0	1	0	15	-.008	4
183	16	max	1120.655	2	1.47	4	.284	1	0	3	.001	1	-.002	15
184		min	-1556.856	3	.339	12	.012	15	0	1	0	15	-.008	4
185	17	max	1121.129	2	1.433	4	.284	1	0	3	.001	1	-.002	15
186		min	-1556.5	3	.325	12	.012	15	0	1	0	15	-.009	4
187	18	max	1121.603	2	1.396	4	.284	1	0	3	.002	1	-.002	15
188		min	-1556.145	3	.31	12	.012	15	0	1	0	15	-.009	4
189	19	max	1122.076	2	1.359	4	.284	1	0	3	.002	1	-.002	15
190		min	-1555.79	3	.296	12	.012	15	0	1	0	15	-.01	4
191	M3	1	max	772.809	2	8.995	4	.139	1	0	5	0	.01	4
192		min	-902.747	3	2.114	15	.006	15	0	1	0	15	.002	15
193	2	max	772.639	2	8.123	4	.139	1	0	5	0	1	.006	2
194		min	-902.875	3	1.909	15	.006	15	0	1	0	15	0	12
195	3	max	772.468	2	7.251	4	.139	1	0	5	0	1	.003	2
196		min	-903.002	3	1.704	15	.006	15	0	1	0	15	0	3
197	4	max	772.298	2	6.379	4	.139	1	0	5	0	1	0	2





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-903.13	3	1.499	15	.006	15	0	1	0	15	-.002	3
199	5	max	772.128	2	5.507	4	.139	1	0	5	0	1	0	15
200		min	-903.258	3	1.294	15	.006	15	0	1	0	15	-.004	3
201	6	max	771.957	2	4.635	4	.139	1	0	5	0	1	-.001	15
202		min	-903.386	3	1.089	15	.006	15	0	1	0	15	-.006	4
203	7	max	771.787	2	3.763	4	.139	1	0	5	0	1	-.002	15
204		min	-903.513	3	.885	15	.006	15	0	1	0	15	-.008	4
205	8	max	771.617	2	2.891	4	.139	1	0	5	0	1	-.002	15
206		min	-903.641	3	.68	15	.006	15	0	1	0	15	-.01	4
207	9	max	771.446	2	2.019	4	.139	1	0	5	0	1	-.003	15
208		min	-903.769	3	.475	15	.006	15	0	1	0	15	-.011	4
209	10	max	771.276	2	1.147	4	.139	1	0	5	0	1	-.003	15
210		min	-903.897	3	.27	15	.006	15	0	1	0	15	-.012	4
211	11	max	771.106	2	.4	2	.139	1	0	5	0	1	-.003	15
212		min	-904.024	3	-.093	3	.006	15	0	1	0	15	-.012	4
213	12	max	770.935	2	-.14	15	.139	1	0	5	0	1	-.003	15
214		min	-904.152	3	-.603	3	.006	15	0	1	0	15	-.012	4
215	13	max	770.765	2	-.345	15	.139	1	0	5	0	1	-.003	15
216		min	-904.28	3	-1.469	4	.006	15	0	1	0	15	-.012	4
217	14	max	770.595	2	-.55	15	.139	1	0	5	0	1	-.003	15
218		min	-904.408	3	-2.341	4	.006	15	0	1	0	15	-.011	4
219	15	max	770.424	2	-.755	15	.139	1	0	5	.001	1	-.002	15
220		min	-904.535	3	-3.213	4	.006	15	0	1	0	15	-.009	4
221	16	max	770.254	2	-.96	15	.139	1	0	5	.001	1	-.002	15
222		min	-904.663	3	-4.085	4	.006	15	0	1	0	15	-.008	4
223	17	max	770.084	2	-1.165	15	.139	1	0	5	.001	1	-.001	15
224		min	-904.791	3	-4.957	4	.006	15	0	1	0	15	-.006	4
225	18	max	769.913	2	-1.37	15	.139	1	0	5	.001	1	0	15
226		min	-904.919	3	-5.829	4	.006	15	0	1	0	15	-.003	4
227	19	max	769.743	2	-1.575	15	.139	1	0	5	.001	1	0	1
228		min	-905.047	3	-6.701	4	.006	15	0	1	0	15	0	1
229	M4	1	max	956.751	1	0	1	-.264	15	0	1	0	1	0
230		min	-156.328	3	0	1	-6.293	1	0	1	0	15	0	1
231	2	max	956.921	1	0	1	-.264	15	0	1	0	1	0	1
232		min	-156.2	3	0	1	-6.293	1	0	1	0	10	0	1
233	3	max	957.091	1	0	1	-.264	15	0	1	0	15	0	1
234		min	-156.072	3	0	1	-6.293	1	0	1	0	1	0	1
235	4	max	957.262	1	0	1	-.264	15	0	1	0	15	0	1
236		min	-155.945	3	0	1	-6.293	1	0	1	-.001	1	0	1
237	5	max	957.432	1	0	1	-.264	15	0	1	0	15	0	1
238		min	-155.817	3	0	1	-6.293	1	0	1	-.002	1	0	1
239	6	max	957.602	1	0	1	-.264	15	0	1	0	15	0	1
240		min	-155.689	3	0	1	-6.293	1	0	1	-.003	1	0	1
241	7	max	957.773	1	0	1	-.264	15	0	1	0	15	0	1
242		min	-155.561	3	0	1	-6.293	1	0	1	-.003	1	0	1
243	8	max	957.943	1	0	1	-.264	15	0	1	0	15	0	1
244		min	-155.434	3	0	1	-6.293	1	0	1	-.004	1	0	1
245	9	max	958.113	1	0	1	-.264	15	0	1	0	15	0	1
246		min	-155.306	3	0	1	-6.293	1	0	1	-.005	1	0	1
247	10	max	958.284	1	0	1	-.264	15	0	1	0	15	0	1
248		min	-155.178	3	0	1	-6.293	1	0	1	-.006	1	0	1
249	11	max	958.454	1	0	1	-.264	15	0	1	0	15	0	1
250		min	-155.05	3	0	1	-6.293	1	0	1	-.006	1	0	1
251	12	max	958.624	1	0	1	-.264	15	0	1	0	15	0	1
252		min	-154.923	3	0	1	-6.293	1	0	1	-.007	1	0	1
253	13	max	958.795	1	0	1	-.264	15	0	1	0	15	0	1
254		min	-154.795	3	0	1	-6.293	1	0	1	-.008	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	958.965	1	0	1	-.264	15	0	1	0	15	0	1
256		min	-154.667	3	0	1	-6.293	1	0	1	-.009	1	0	1
257	15	max	959.135	1	0	1	-.264	15	0	1	0	15	0	1
258		min	-154.539	3	0	1	-6.293	1	0	1	-.009	1	0	1
259	16	max	959.306	1	0	1	-.264	15	0	1	0	15	0	1
260		min	-154.412	3	0	1	-6.293	1	0	1	-.01	1	0	1
261	17	max	959.476	1	0	1	-.264	15	0	1	0	15	0	1
262		min	-154.284	3	0	1	-6.293	1	0	1	-.011	1	0	1
263	18	max	959.646	1	0	1	-.264	15	0	1	0	15	0	1
264		min	-154.156	3	0	1	-6.293	1	0	1	-.011	1	0	1
265	19	max	959.817	1	0	1	-.264	15	0	1	0	15	0	1
266		min	-154.028	3	0	1	-6.293	1	0	1	-.012	1	0	1
267	M6	1	max	3214.494	2	2.405	2	0	1	0	0	1	0	1
268		min	-4670.996	3	.107	3	0	1	0	1	0	1	0	1
269	2	max	3214.968	2	2.376	2	0	1	0	1	0	1	0	3
270		min	-4670.64	3	.085	3	0	1	0	1	0	1	0	2
271	3	max	3215.442	2	2.347	2	0	1	0	1	0	1	0	3
272		min	-4670.285	3	.064	3	0	1	0	1	0	1	-.002	2
273	4	max	3215.915	2	2.318	2	0	1	0	1	0	1	0	3
274		min	-4669.93	3	.042	3	0	1	0	1	0	1	-.002	2
275	5	max	3216.389	2	2.289	2	0	1	0	1	0	1	0	3
276		min	-4669.574	3	.02	3	0	1	0	1	0	1	-.003	2
277	6	max	3216.863	2	2.261	2	0	1	0	1	0	1	0	3
278		min	-4669.219	3	-.001	3	0	1	0	1	0	1	-.004	2
279	7	max	3217.337	2	2.232	2	0	1	0	1	0	1	0	3
280		min	-4668.864	3	-.023	3	0	1	0	1	0	1	-.004	2
281	8	max	3217.81	2	2.203	2	0	1	0	1	0	1	0	3
282		min	-4668.508	3	-.045	3	0	1	0	1	0	1	-.005	2
283	9	max	3218.284	2	2.174	2	0	1	0	1	0	1	0	3
284		min	-4668.153	3	-.066	3	0	1	0	1	0	1	-.006	2
285	10	max	3218.758	2	2.145	2	0	1	0	1	0	1	0	3
286		min	-4667.798	3	-.088	3	0	1	0	1	0	1	-.007	2
287	11	max	3219.232	2	2.116	2	0	1	0	1	0	1	0	3
288		min	-4667.443	3	-.109	3	0	1	0	1	0	1	-.007	2
289	12	max	3219.705	2	2.087	2	0	1	0	1	0	1	0	3
290		min	-4667.087	3	-.131	3	0	1	0	1	0	1	-.008	2
291	13	max	3220.179	2	2.059	2	0	1	0	1	0	1	0	3
292		min	-4666.732	3	-.153	3	0	1	0	1	0	1	-.009	2
293	14	max	3220.653	2	2.03	2	0	1	0	1	0	1	0	3
294		min	-4666.377	3	-.174	3	0	1	0	1	0	1	-.009	2
295	15	max	3221.127	2	2.001	2	0	1	0	1	0	1	0	3
296		min	-4666.021	3	-.196	3	0	1	0	1	0	1	-.01	2
297	16	max	3221.6	2	1.972	2	0	1	0	1	0	1	0	3
298		min	-4665.666	3	-.218	3	0	1	0	1	0	1	-.011	2
299	17	max	3222.074	2	1.943	2	0	1	0	1	0	1	0	3
300		min	-4665.311	3	-.239	3	0	1	0	1	0	1	-.011	2
301	18	max	3222.548	2	1.914	2	0	1	0	1	0	1	0	3
302		min	-4664.955	3	-.261	3	0	1	0	1	0	1	-.012	2
303	19	max	3223.022	2	1.885	2	0	1	0	1	0	1	0	3
304		min	-4664.6	3	-.283	3	0	1	0	1	0	1	-.012	2
305	M7	1	max	2365.573	2	9.015	4	0	1	0	0	1	.012	2
306		min	-2554.167	3	2.117	15	0	1	0	1	0	1	0	3
307	2	max	2365.402	2	8.143	4	0	1	0	1	0	1	.009	2
308		min	-2554.295	3	1.912	15	0	1	0	1	0	1	-.003	3
309	3	max	2365.232	2	7.271	4	0	1	0	1	0	1	.006	2
310		min	-2554.422	3	1.707	15	0	1	0	1	0	1	-.004	3
311	4	max	2365.062	2	6.399	4	0	1	0	1	0	1	.003	2



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2554.55	3	1.502	15	0	1	0	1	0	1	-.006	3
313	5	max	2364.891	2	5.527	4	0	1	0	1	0	1	0	2
314		min	-2554.678	3	1.297	15	0	1	0	1	0	1	-.007	3
315	6	max	2364.721	2	4.655	4	0	1	0	1	0	1	-.001	15
316		min	-2554.806	3	1.092	15	0	1	0	1	0	1	-.008	3
317	7	max	2364.551	2	3.783	4	0	1	0	1	0	1	-.002	15
318		min	-2554.933	3	.887	15	0	1	0	1	0	1	-.009	3
319	8	max	2364.38	2	2.911	4	0	1	0	1	0	1	-.002	15
320		min	-2555.061	3	.67	12	0	1	0	1	0	1	-.01	4
321	9	max	2364.21	2	2.133	2	0	1	0	1	0	1	-.003	15
322		min	-2555.189	3	.33	12	0	1	0	1	0	1	-.011	4
323	10	max	2364.039	2	1.453	2	0	1	0	1	0	1	-.003	15
324		min	-2555.317	3	-.059	3	0	1	0	1	0	1	-.012	4
325	11	max	2363.869	2	.774	2	0	1	0	1	0	1	-.003	15
326		min	-2555.445	3	-.569	3	0	1	0	1	0	1	-.012	4
327	12	max	2363.699	2	.094	2	0	1	0	1	0	1	-.003	15
328		min	-2555.572	3	-1.079	3	0	1	0	1	0	1	-.012	4
329	13	max	2363.528	2	-.342	15	0	1	0	1	0	1	-.003	15
330		min	-2555.7	3	-1.588	3	0	1	0	1	0	1	-.012	4
331	14	max	2363.358	2	-.547	15	0	1	0	1	0	1	-.003	15
332		min	-2555.828	3	-2.321	4	0	1	0	1	0	1	-.011	4
333	15	max	2363.188	2	-.752	15	0	1	0	1	0	1	-.002	15
334		min	-2555.956	3	-3.193	4	0	1	0	1	0	1	-.009	4
335	16	max	2363.017	2	-.957	15	0	1	0	1	0	1	-.002	15
336		min	-2556.083	3	-4.065	4	0	1	0	1	0	1	-.008	4
337	17	max	2362.847	2	-1.162	15	0	1	0	1	0	1	-.001	15
338		min	-2556.211	3	-4.937	4	0	1	0	1	0	1	-.005	4
339	18	max	2362.677	2	-1.367	15	0	1	0	1	0	1	0	15
340		min	-2556.339	3	-5.809	4	0	1	0	1	0	1	-.003	4
341	19	max	2362.506	2	-1.572	15	0	1	0	1	0	1	0	1
342		min	-2556.467	3	-6.681	4	0	1	0	1	0	1	0	1
343	M8	1	max	2722.19	2	0	1	0	1	0	1	0	1	1
344		min	-550.056	3	0	1	0	1	0	1	0	1	0	1
345	2	max	2722.361	2	0	1	0	1	0	1	0	1	0	1
346		min	-549.928	3	0	1	0	1	0	1	0	1	0	1
347	3	max	2722.531	2	0	1	0	1	0	1	0	1	0	1
348		min	-549.801	3	0	1	0	1	0	1	0	1	0	1
349	4	max	2722.701	2	0	1	0	1	0	1	0	1	0	1
350		min	-549.673	3	0	1	0	1	0	1	0	1	0	1
351	5	max	2722.872	2	0	1	0	1	0	1	0	1	0	1
352		min	-549.545	3	0	1	0	1	0	1	0	1	0	1
353	6	max	2723.042	2	0	1	0	1	0	1	0	1	0	1
354		min	-549.417	3	0	1	0	1	0	1	0	1	0	1
355	7	max	2723.212	2	0	1	0	1	0	1	0	1	0	1
356		min	-549.29	3	0	1	0	1	0	1	0	1	0	1
357	8	max	2723.383	2	0	1	0	1	0	1	0	1	0	1
358		min	-549.162	3	0	1	0	1	0	1	0	1	0	1
359	9	max	2723.553	2	0	1	0	1	0	1	0	1	0	1
360		min	-549.034	3	0	1	0	1	0	1	0	1	0	1
361	10	max	2723.723	2	0	1	0	1	0	1	0	1	0	1
362		min	-548.906	3	0	1	0	1	0	1	0	1	0	1
363	11	max	2723.894	2	0	1	0	1	0	1	0	1	0	1
364		min	-548.779	3	0	1	0	1	0	1	0	1	0	1
365	12	max	2724.064	2	0	1	0	1	0	1	0	1	0	1
366		min	-548.651	3	0	1	0	1	0	1	0	1	0	1
367	13	max	2724.234	2	0	1	0	1	0	1	0	1	0	1
368		min	-548.523	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2724.405	2	0	1	0	1	0	1	0	1	0	1
370			min	-548.395	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2724.575	2	0	1	0	1	0	1	0	1	0	1
372			min	-548.268	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2724.745	2	0	1	0	1	0	1	0	1	0	1
374			min	-548.14	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2724.916	2	0	1	0	1	0	1	0	1	0	1
376			min	-548.012	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2725.086	2	0	1	0	1	0	1	0	1	0	1
378			min	-547.884	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2725.257	2	0	1	0	1	0	1	0	1	0	1
380			min	-547.756	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1113.549	2	2.026	4	-.012	15	0	1	0	2	0	1
382			min	-1562.185	3	.476	15	-.284	1	0	3	0	3	0	1
383		2	max	1114.023	2	1.989	4	-.012	15	0	1	0	10	0	15
384			min	-1561.83	3	.468	15	-.284	1	0	3	0	1	0	4
385		3	max	1114.497	2	1.952	4	-.012	15	0	1	0	10	0	15
386			min	-1561.475	3	.459	15	-.284	1	0	3	0	1	-.001	4
387		4	max	1114.97	2	1.915	4	-.012	15	0	1	0	10	0	15
388			min	-1561.119	3	.45	15	-.284	1	0	3	0	1	-.002	4
389		5	max	1115.444	2	1.878	4	-.012	15	0	1	0	10	0	15
390			min	-1560.764	3	.441	15	-.284	1	0	3	0	1	-.002	4
391		6	max	1115.918	2	1.841	4	-.012	15	0	1	0	15	0	15
392			min	-1560.409	3	.433	15	-.284	1	0	3	0	1	-.003	4
393		7	max	1116.391	2	1.804	4	-.012	15	0	1	0	15	0	15
394			min	-1560.053	3	.424	15	-.284	1	0	3	0	1	-.004	4
395		8	max	1116.865	2	1.767	4	-.012	15	0	1	0	15	0	15
396			min	-1559.698	3	.415	15	-.284	1	0	3	0	1	-.004	4
397		9	max	1117.339	2	1.73	4	-.012	15	0	1	0	15	-.001	15
398			min	-1559.343	3	.407	15	-.284	1	0	3	0	1	-.005	4
399		10	max	1117.813	2	1.693	4	-.012	15	0	1	0	15	-.001	15
400			min	-1558.987	3	.398	15	-.284	1	0	3	0	1	-.005	4
401		11	max	1118.286	2	1.656	4	-.012	15	0	1	0	15	-.001	15
402			min	-1558.632	3	.389	15	-.284	1	0	3	0	1	-.006	4
403		12	max	1118.76	2	1.619	4	-.012	15	0	1	0	15	-.002	15
404			min	-1558.277	3	.38	15	-.284	1	0	3	0	1	-.006	4
405		13	max	1119.234	2	1.581	4	-.012	15	0	1	0	15	-.002	15
406			min	-1557.921	3	.372	15	-.284	1	0	3	-.001	1	-.007	4
407		14	max	1119.708	2	1.544	4	-.012	15	0	1	0	15	-.002	15
408			min	-1557.566	3	.363	15	-.284	1	0	3	-.001	1	-.007	4
409		15	max	1120.181	2	1.507	4	-.012	15	0	1	0	15	-.002	15
410			min	-1557.211	3	.354	12	-.284	1	0	3	-.001	1	-.008	4
411		16	max	1120.655	2	1.47	4	-.012	15	0	1	0	15	-.002	15
412			min	-1556.856	3	.339	12	-.284	1	0	3	-.001	1	-.008	4
413		17	max	1121.129	2	1.433	4	-.012	15	0	1	0	15	-.002	15
414			min	-1556.5	3	.325	12	-.284	1	0	3	-.001	1	-.009	4
415		18	max	1121.603	2	1.396	4	-.012	15	0	1	0	15	-.002	15
416			min	-1556.145	3	.31	12	-.284	1	0	3	-.002	1	-.009	4
417		19	max	1122.076	2	1.359	4	-.012	15	0	1	0	15	-.002	15
418			min	-1555.79	3	.296	12	-.284	1	0	3	-.002	1	-.01	4
419	M11	1	max	772.809	2	8.995	4	-.006	15	0	1	0	15	.01	4
420			min	-902.747	3	2.114	15	-.139	1	0	5	0	1	.002	15
421		2	max	772.639	2	8.123	4	-.006	15	0	1	0	15	.006	2
422			min	-902.875	3	1.909	15	-.139	1	0	5	0	1	0	12
423		3	max	772.468	2	7.251	4	-.006	15	0	1	0	15	.003	2
424			min	-903.002	3	1.704	15	-.139	1	0	5	0	1	0	3
425		4	max	772.298	2	6.379	4	-.006	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-903.13	3	1.499	15	-.139	1	0	5	0	1	-.002	3
427		5	max	772.128	2	5.507	4	-.006	15	0	1	0	15	0	15
428			min	-903.258	3	1.294	15	-.139	1	0	5	0	1	-.004	3
429		6	max	771.957	2	4.635	4	-.006	15	0	1	0	15	-.001	15
430			min	-903.386	3	1.089	15	-.139	1	0	5	0	1	-.006	4
431		7	max	771.787	2	3.763	4	-.006	15	0	1	0	15	-.002	15
432			min	-903.513	3	.885	15	-.139	1	0	5	0	1	-.008	4
433		8	max	771.617	2	2.891	4	-.006	15	0	1	0	15	-.002	15
434			min	-903.641	3	.68	15	-.139	1	0	5	0	1	-.01	4
435		9	max	771.446	2	2.019	4	-.006	15	0	1	0	15	-.003	15
436			min	-903.769	3	.475	15	-.139	1	0	5	0	1	-.011	4
437		10	max	771.276	2	1.147	4	-.006	15	0	1	0	15	-.003	15
438			min	-903.897	3	.27	15	-.139	1	0	5	0	1	-.012	4
439		11	max	771.106	2	.4	2	-.006	15	0	1	0	15	-.003	15
440			min	-904.024	3	-.093	3	-.139	1	0	5	0	1	-.012	4
441		12	max	770.935	2	-.14	15	-.006	15	0	1	0	15	-.003	15
442			min	-904.152	3	-.603	3	-.139	1	0	5	0	1	-.012	4
443		13	max	770.765	2	-.345	15	-.006	15	0	1	0	15	-.003	15
444			min	-904.28	3	-1.469	4	-.139	1	0	5	0	1	-.012	4
445		14	max	770.595	2	-.55	15	-.006	15	0	1	0	15	-.003	15
446			min	-904.408	3	-2.341	4	-.139	1	0	5	0	1	-.011	4
447		15	max	770.424	2	-.755	15	-.006	15	0	1	0	15	-.002	15
448			min	-904.535	3	-3.213	4	-.139	1	0	5	-.001	1	-.009	4
449		16	max	770.254	2	-.96	15	-.006	15	0	1	0	15	-.002	15
450			min	-904.663	3	-4.085	4	-.139	1	0	5	-.001	1	-.008	4
451		17	max	770.084	2	-1.165	15	-.006	15	0	1	0	15	-.001	15
452			min	-904.791	3	-4.957	4	-.139	1	0	5	-.001	1	-.006	4
453		18	max	769.913	2	-1.37	15	-.006	15	0	1	0	15	0	15
454			min	-904.919	3	-5.829	4	-.139	1	0	5	-.001	1	-.003	4
455		19	max	769.743	2	-1.575	15	-.006	15	0	1	0	15	0	1
456			min	-905.047	3	-6.701	4	-.139	1	0	5	-.001	1	0	1
457	M12	1	max	956.751	1	0	1	6.293	1	0	1	0	15	0	1
458			min	-156.328	3	0	1	.264	15	0	1	0	1	0	1
459		2	max	956.921	1	0	1	6.293	1	0	1	0	10	0	1
460			min	-156.2	3	0	1	.264	15	0	1	0	1	0	1
461		3	max	957.091	1	0	1	6.293	1	0	1	0	1	0	1
462			min	-156.072	3	0	1	.264	15	0	1	0	15	0	1
463		4	max	957.262	1	0	1	6.293	1	0	1	.001	1	0	1
464			min	-155.945	3	0	1	.264	15	0	1	0	15	0	1
465		5	max	957.432	1	0	1	6.293	1	0	1	.002	1	0	1
466			min	-155.817	3	0	1	.264	15	0	1	0	15	0	1
467		6	max	957.602	1	0	1	6.293	1	0	1	.003	1	0	1
468			min	-155.689	3	0	1	.264	15	0	1	0	15	0	1
469		7	max	957.773	1	0	1	6.293	1	0	1	.003	1	0	1
470			min	-155.561	3	0	1	.264	15	0	1	0	15	0	1
471		8	max	957.943	1	0	1	6.293	1	0	1	.004	1	0	1
472			min	-155.434	3	0	1	.264	15	0	1	0	15	0	1
473		9	max	958.113	1	0	1	6.293	1	0	1	.005	1	0	1
474			min	-155.306	3	0	1	.264	15	0	1	0	15	0	1
475		10	max	958.284	1	0	1	6.293	1	0	1	.006	1	0	1
476			min	-155.178	3	0	1	.264	15	0	1	0	15	0	1
477		11	max	958.454	1	0	1	6.293	1	0	1	.006	1	0	1
478			min	-155.05	3	0	1	.264	15	0	1	0	15	0	1
479		12	max	958.624	1	0	1	6.293	1	0	1	.007	1	0	1
480			min	-154.923	3	0	1	.264	15	0	1	0	15	0	1
481		13	max	958.795	1	0	1	6.293	1	0	1	.008	1	0	1
482			min	-154.795	3	0	1	.264	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	958.965	1	0	1	6.293	1	0	1	.009	1	0	1
484			min	-154.667	3	0	1	.264	15	0	1	0	15	0	1
485		15	max	959.135	1	0	1	6.293	1	0	1	.009	1	0	1
486			min	-154.539	3	0	1	.264	15	0	1	0	15	0	1
487		16	max	959.306	1	0	1	6.293	1	0	1	.01	1	0	1
488			min	-154.412	3	0	1	.264	15	0	1	0	15	0	1
489		17	max	959.476	1	0	1	6.293	1	0	1	.011	1	0	1
490			min	-154.284	3	0	1	.264	15	0	1	0	15	0	1
491		18	max	959.646	1	0	1	6.293	1	0	1	.011	1	0	1
492			min	-154.156	3	0	1	.264	15	0	1	0	15	0	1
493		19	max	959.817	1	0	1	6.293	1	0	1	.012	1	0	1
494			min	-154.028	3	0	1	.264	15	0	1	0	15	0	1
495	M1	1	max	124.41	1	818.108	3	-1.535	15	0	2	.1	1	0	15
496			min	5.091	15	-433.481	2	-36.082	1	0	3	.004	15	-.012	3
497		2	max	125.122	1	816.963	3	-1.535	15	0	2	.078	1	.26	2
498			min	5.306	15	-435.008	2	-36.082	1	0	3	.003	15	-.52	3
499		3	max	587.296	3	593.209	2	-1.523	15	0	3	.056	1	.519	2
500			min	-358.072	2	-644.211	3	-35.878	1	0	2	.002	15	-1.01	3
501		4	max	587.83	3	591.682	2	-1.523	15	0	3	.033	1	.151	2
502			min	-357.36	2	-645.356	3	-35.878	1	0	2	.001	15	-.61	3
503		5	max	588.364	3	590.155	2	-1.523	15	0	3	.011	1	-.005	15
504			min	-356.648	2	-646.501	3	-35.878	1	0	2	0	10	-.216	2
505		6	max	588.898	3	588.628	2	-1.523	15	0	3	0	15	.192	3
506			min	-355.936	2	-647.646	3	-35.878	1	0	2	-.011	1	-.581	2
507		7	max	589.432	3	587.101	2	-1.523	15	0	3	-.001	15	.595	3
508			min	-355.224	2	-648.791	3	-35.878	1	0	2	-.034	1	-.946	2
509		8	max	589.966	3	585.574	2	-1.523	15	0	3	-.002	15	.998	3
510			min	-354.512	2	-649.937	3	-35.878	1	0	2	-.056	1	-1.31	2
511		9	max	604.071	3	48.363	2	-2.642	15	0	9	.038	1	1.161	3
512			min	-304.197	2	.465	15	-62.505	1	0	3	.002	15	-1.49	2
513		10	max	604.605	3	46.836	2	-2.642	15	0	9	0	10	1.138	3
514			min	-303.485	2	.005	15	-62.505	1	0	3	0	1	-1.52	2
515		11	max	605.139	3	45.309	2	-2.642	15	0	9	-.002	15	1.115	3
516			min	-302.773	2	-1.883	4	-62.505	1	0	3	-.039	1	-1.549	2
517		12	max	618.772	3	432.368	3	-1.468	15	0	2	.055	1	.982	3
518			min	-252.233	2	-690.051	2	-34.951	1	0	3	.002	15	-1.376	2
519		13	max	619.306	3	431.222	3	-1.468	15	0	2	.033	1	.714	3
520			min	-251.521	2	-691.578	2	-34.951	1	0	3	.001	15	-.948	2
521		14	max	619.84	3	430.077	3	-1.468	15	0	2	.012	1	.446	3
522			min	-250.809	2	-693.105	2	-34.951	1	0	3	0	15	-.518	2
523		15	max	620.374	3	428.932	3	-1.468	15	0	2	0	15	.18	3
524			min	-250.097	2	-694.632	2	-34.951	1	0	3	-.01	1	-.099	1
525		16	max	620.908	3	427.787	3	-1.468	15	0	2	-.001	15	.344	2
526			min	-249.385	2	-696.159	2	-34.951	1	0	3	-.032	1	-.086	3
527		17	max	621.442	3	426.642	3	-1.468	15	0	2	-.002	15	.777	2
528			min	-248.673	2	-697.686	2	-34.951	1	0	3	-.053	1	-.351	3
529		18	max	-5.32	15	639.366	2	-1.726	15	0	3	-.003	15	.394	2
530			min	-125.851	1	-297.7	3	-40.957	1	0	2	-.077	1	-.175	3
531		19	max	-5.105	15	637.839	2	-1.726	15	0	3	-.004	15	.011	3
532			min	-125.139	1	-298.845	3	-40.957	1	0	2	-.103	1	-.003	1
533	M5	1	max	289.481	1	2645.461	3	0	1	0	1	0	1	.025	3
534			min	5.222	12	-1510.479	2	0	1	0	1	0	1	0	15
535		2	max	290.193	1	2644.316	3	0	1	0	1	0	1	.958	2
536			min	5.578	12	-1512.006	2	0	1	0	1	0	1	-1.617	3
537		3	max	1718.371	3	1441.136	2	0	1	0	1	0	1	1.864	2
538			min	-1053.489	2	-1743.073	3	0	1	0	1	0	1	-3.21	3
539		4	max	1718.905	3	1439.609	2	0	1	0	1	0	1	.97	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1052.777	2	-1744.218	3	0	1	0	1	0	1	-2.128	3
541		5	max	1719.439	3	1438.083	2	0	1	0	1	0	1	.107	1
542			min	-1052.065	2	-1745.364	3	0	1	0	1	0	1	-1.045	3
543		6	max	1719.973	3	1436.556	2	0	1	0	1	0	1	.039	3
544			min	-1051.353	2	-1746.509	3	0	1	0	1	0	1	-.815	2
545		7	max	1720.507	3	1435.029	2	0	1	0	1	0	1	1.123	3
546			min	-1050.641	2	-1747.654	3	0	1	0	1	0	1	-1.706	2
547		8	max	1721.041	3	1433.502	2	0	1	0	1	0	1	2.208	3
548			min	-1049.929	2	-1748.799	3	0	1	0	1	0	1	-2.596	2
549		9	max	1727.516	3	166.699	2	0	1	0	1	0	1	2.552	3
550			min	-930.751	2	.458	15	0	1	0	1	0	1	-2.982	2
551		10	max	1728.05	3	165.173	2	0	1	0	1	0	1	2.453	3
552			min	-930.039	2	-.003	15	0	1	0	1	0	1	-3.085	2
553		11	max	1728.584	3	163.646	2	0	1	0	1	0	1	2.354	3
554			min	-929.327	2	-1.835	4	0	1	0	1	0	1	-3.187	2
555		12	max	1736.002	3	1091.333	3	0	1	0	1	0	1	2.05	3
556			min	-810.598	2	-1762.547	2	0	1	0	1	0	1	-2.843	2
557		13	max	1736.536	3	1090.188	3	0	1	0	1	0	1	1.373	3
558			min	-809.886	2	-1764.073	2	0	1	0	1	0	1	-1.749	2
559		14	max	1737.07	3	1089.042	3	0	1	0	1	0	1	.697	3
560			min	-809.174	2	-1765.6	2	0	1	0	1	0	1	-.654	2
561		15	max	1737.604	3	1087.897	3	0	1	0	1	0	1	.442	2
562			min	-808.462	2	-1767.127	2	0	1	0	1	0	1	.001	15
563		16	max	1738.138	3	1086.752	3	0	1	0	1	0	1	1.54	2
564			min	-807.75	2	-1768.654	2	0	1	0	1	0	1	-.654	3
565		17	max	1738.672	3	1085.607	3	0	1	0	1	0	1	2.638	2
566			min	-807.038	2	-1770.181	2	0	1	0	1	0	1	-1.328	3
567		18	max	-7.409	12	2156.218	2	0	1	0	1	0	1	1.342	2
568			min	-288.735	1	-1071.852	3	0	1	0	1	0	1	-.687	3
569		19	max	-7.053	12	2154.691	2	0	1	0	1	0	1	.007	1
570			min	-288.023	1	-1072.997	3	0	1	0	1	0	1	-.021	3
571	M9	1	max	124.41	1	818.108	3	36.082	1	0	3	-.004	15	0	15
572			min	5.091	15	-433.481	2	1.535	15	0	2	-.1	1	-.012	3
573		2	max	125.122	1	816.963	3	36.082	1	0	3	-.003	15	.26	2
574			min	5.306	15	-435.008	2	1.535	15	0	2	-.078	1	-.52	3
575		3	max	587.296	3	593.209	2	35.878	1	0	2	-.002	15	.519	2
576			min	-358.072	2	-644.211	3	1.523	15	0	3	-.056	1	-1.01	3
577		4	max	587.83	3	591.682	2	35.878	1	0	2	-.001	15	.151	2
578			min	-357.36	2	-645.356	3	1.523	15	0	3	-.033	1	-.61	3
579		5	max	588.364	3	590.155	2	35.878	1	0	2	0	10	-.005	15
580			min	-356.648	2	-646.501	3	1.523	15	0	3	-.011	1	-.216	2
581		6	max	588.898	3	588.628	2	35.878	1	0	2	.011	1	.192	3
582			min	-355.936	2	-647.646	3	1.523	15	0	3	0	15	-.581	2
583		7	max	589.432	3	587.101	2	35.878	1	0	2	.034	1	.595	3
584			min	-355.224	2	-648.791	3	1.523	15	0	3	.001	15	-.946	2
585		8	max	589.966	3	585.574	2	35.878	1	0	2	.056	1	.998	3
586			min	-354.512	2	-649.937	3	1.523	15	0	3	.002	15	-1.31	2
587		9	max	604.071	3	48.363	2	62.505	1	0	3	-.002	15	1.161	3
588			min	-304.197	2	.465	15	2.642	15	0	9	-.038	1	-1.49	2
589		10	max	604.605	3	46.836	2	62.505	1	0	3	0	1	1.138	3
590			min	-303.485	2	.005	15	2.642	15	0	9	0	10	-1.52	2
591		11	max	605.139	3	45.309	2	62.505	1	0	3	.039	1	1.115	3
592			min	-302.773	2	-1.883	4	2.642	15	0	9	.002	15	-1.549	2
593		12	max	618.772	3	432.368	3	34.951	1	0	3	-.002	15	.982	3
594			min	-252.233	2	-690.051	2	1.468	15	0	2	-.055	1	-1.376	2
595		13	max	619.306	3	431.222	3	34.951	1	0	3	-.001	15	.714	3
596			min	-251.521	2	-691.578	2	1.468	15	0	2	-.033	1	-.948	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	619.84	3	430.077	3	34.951	1	0	3	0	15	.446	3
598		min	-250.809	2	-693.105	2	1.468	15	0	2	-.012	1	-.518	2
599	15	max	620.374	3	428.932	3	34.951	1	0	3	.01	1	.18	3
600		min	-250.097	2	-694.632	2	1.468	15	0	2	0	15	-.099	1
601	16	max	620.908	3	427.787	3	34.951	1	0	3	.032	1	.344	2
602		min	-249.385	2	-696.159	2	1.468	15	0	2	.001	15	-.086	3
603	17	max	621.442	3	426.642	3	34.951	1	0	3	.053	1	.777	2
604		min	-248.673	2	-697.686	2	1.468	15	0	2	.002	15	-.351	3
605	18	max	-5.32	15	639.366	2	40.957	1	0	2	.077	1	.394	2
606		min	-125.851	1	-297.7	3	1.726	15	0	3	.003	15	-.175	3
607	19	max	-5.105	15	637.839	2	40.957	1	0	2	.103	1	.011	3
608		min	-125.139	1	-298.845	3	1.726	15	0	3	.004	15	-.003	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.239	2	.011	3	1.635e-2	2	NC	1	NC	1
2			min	0	15	-.074	3	-.007	2	-4.926e-3	3	NC	1	NC	1
3		2	max	0	1	.201	2	.013	3	1.695e-2	2	NC	4	NC	1
4			min	0	15	.004	15	-.005	2	-4.176e-3	3	1379.55	3	NC	1
5		3	max	0	1	.172	2	.017	1	1.756e-2	2	NC	4	NC	2
6			min	0	15	.004	15	-.004	10	-3.426e-3	3	752.234	3	7974.855	1
7		4	max	0	1	.185	3	.025	1	1.816e-2	2	NC	4	NC	2
8			min	0	15	.003	15	-.005	10	-2.676e-3	3	577.45	3	5562.229	1
9		5	max	0	1	.21	3	.029	1	1.876e-2	2	NC	4	NC	2
10			min	0	15	.003	15	-.005	10	-1.925e-3	3	528.425	3	4962.186	1
11		6	max	0	1	.198	3	.026	3	1.936e-2	2	NC	4	NC	2
12			min	0	15	.004	15	-.007	10	-1.175e-3	3	551.294	3	5446.971	1
13		7	max	0	1	.225	2	.029	3	1.996e-2	2	NC	2	NC	2
14			min	0	15	.004	15	-.009	10	-4.253e-4	3	648.957	3	7716.241	1
15		8	max	0	1	.268	2	.031	3	2.057e-2	2	NC	4	NC	1
16			min	0	15	.005	15	-.015	2	3.247e-4	3	861.829	3	7768.506	3
17		9	max	0	1	.305	2	.032	3	2.117e-2	2	NC	4	NC	1
18			min	0	15	.006	15	-.02	2	4.052e-4	15	1248.341	3	7362.902	3
19		10	max	0	1	.321	2	.032	3	2.177e-2	2	NC	4	NC	1
20		min	0	1	.006	15	-.022	2	4.119e-4	15	1576.577	3	7244.26	3	
21	11	max	0	15	.305	2	.032	3	2.117e-2	2	NC	4	NC	1	
22		min	0	1	.006	15	-.02	2	4.052e-4	15	1248.341	3	7362.902	3	
23	12	max	0	15	.268	2	.031	3	2.057e-2	2	NC	4	NC	1	
24		min	0	1	.005	15	-.015	2	3.247e-4	3	861.829	3	7768.506	3	
25	13	max	0	15	.225	2	.029	3	1.996e-2	2	NC	2	NC	2	
26		min	0	1	.004	15	-.009	10	-4.253e-4	3	648.957	3	7716.241	1	
27	14	max	0	15	.198	3	.026	3	1.936e-2	2	NC	4	NC	2	
28		min	0	1	.004	15	-.007	10	-1.175e-3	3	551.294	3	5446.971	1	
29	15	max	0	15	.21	3	.029	1	1.876e-2	2	NC	4	NC	2	
30		min	0	1	.003	15	-.005	10	-1.925e-3	3	528.425	3	4962.186	1	
31	16	max	0	15	.185	3	.025	1	1.816e-2	2	NC	4	NC	2	
32		min	0	1	.003	15	-.005	10	-2.676e-3	3	577.45	3	5562.229	1	
33	17	max	0	15	.172	2	.017	1	1.756e-2	2	NC	4	NC	2	
34		min	0	1	.004	15	-.004	10	-3.426e-3	3	752.234	3	7974.855	1	
35	18	max	0	15	.201	2	.013	3	1.695e-2	2	NC	4	NC	1	
36		min	0	1	.004	15	-.005	2	-4.176e-3	3	1379.55	3	NC	1	
37	19	max	0	15	.239	2	.011	3	1.635e-2	2	NC	1	NC	1	
38		min	0	1	-.074	3	-.007	2	-4.926e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.49	3	.01	3	8.847e-3	2	NC	1	NC	1
40			min	0	15	-.695	2	-.007	2	-7.322e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.641	3	.011	3	9.856e-3	2	NC	5	NC	1
42			min	0	15	-.848	2	-.005	2	-8.273e-3	3	977.13	2	NC	1
43		3	max	0	1	.776	3	.014	3	1.086e-2	2	NC	5	NC	1
44			min	0	15	-.989	2	-.004	10	-9.224e-3	3	509.037	2	NC	1
45		4	max	0	1	.885	3	.02	1	1.187e-2	2	NC	5	NC	2
46			min	0	15	-1.109	2	-.004	10	-1.018e-2	3	361.998	2	7002.667	1
47		5	max	0	1	.961	3	.024	1	1.288e-2	2	NC	5	NC	2
48			min	0	15	-1.202	2	-.005	10	-1.113e-2	3	295.869	2	5939.323	1
49		6	max	0	1	1.004	3	.023	3	1.389e-2	2	NC	15	NC	2
50			min	0	15	-1.265	2	-.006	10	-1.208e-2	3	262.961	2	6305.075	1
51		7	max	0	1	1.016	3	.025	3	1.49e-2	2	NC	15	NC	2
52			min	0	15	-1.301	2	-.008	10	-1.303e-2	3	247.471	2	8704.632	1
53		8	max	0	1	1.005	3	.027	3	1.591e-2	2	NC	15	NC	1
54			min	0	15	-1.314	2	-.013	2	-1.398e-2	3	242.31	2	8890.298	3
55		9	max	0	1	.986	3	.028	3	1.692e-2	2	NC	15	NC	1
56			min	0	15	-1.312	2	-.018	2	-1.493e-2	3	242.871	2	8351.676	3
57		10	max	0	1	.974	3	.028	3	1.793e-2	2	NC	15	NC	1
58			min	0	1	-1.309	2	-.02	2	-1.588e-2	3	244.369	2	8192.016	3
59		11	max	0	15	.986	3	.028	3	1.692e-2	2	NC	15	NC	1
60			min	0	1	-1.312	2	-.018	2	-1.493e-2	3	242.871	2	8351.676	3
61		12	max	0	15	1.005	3	.027	3	1.591e-2	2	NC	15	NC	1
62			min	0	1	-1.314	2	-.013	2	-1.398e-2	3	242.31	2	8890.298	3
63		13	max	0	15	1.016	3	.025	3	1.49e-2	2	NC	15	NC	2
64			min	0	1	-1.301	2	-.008	10	-1.303e-2	3	247.471	2	8704.632	1
65		14	max	0	15	1.004	3	.023	3	1.389e-2	2	NC	15	NC	2
66			min	0	1	-1.265	2	-.006	10	-1.208e-2	3	262.961	2	6305.075	1
67		15	max	0	15	.961	3	.024	1	1.288e-2	2	NC	5	NC	2
68			min	0	1	-1.202	2	-.005	10	-1.113e-2	3	295.869	2	5939.323	1
69		16	max	0	15	.885	3	.02	1	1.187e-2	2	NC	5	NC	2
70			min	0	1	-1.109	2	-.004	10	-1.018e-2	3	361.998	2	7002.667	1
71		17	max	0	15	.776	3	.014	3	1.086e-2	2	NC	5	NC	1
72			min	0	1	-.989	2	-.004	10	-9.224e-3	3	509.037	2	NC	1
73		18	max	0	15	.641	3	.011	3	9.856e-3	2	NC	5	NC	1
74			min	0	1	-.848	2	-.005	2	-8.273e-3	3	977.13	2	NC	1
75		19	max	0	15	.49	3	.01	3	8.847e-3	2	NC	1	NC	1
76			min	0	1	-.695	2	-.007	2	-7.322e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.502	3	.009	3	6.18e-3	3	NC	1	NC	1
78			min	0	1	-.693	2	-.006	2	-9.183e-3	2	NC	1	NC	1
79		2	max	0	15	.62	3	.01	3	6.963e-3	3	NC	5	NC	1
80			min	0	1	-.869	2	-.005	2	-1.024e-2	2	850.523	2	NC	1
81		3	max	0	15	.73	3	.013	1	7.747e-3	3	NC	5	NC	1
82			min	0	1	-1.029	2	-.004	10	-1.129e-2	2	446.346	2	NC	1
83		4	max	0	15	.824	3	.02	1	8.53e-3	3	NC	5	NC	2
84			min	0	1	-1.16	2	-.004	10	-1.235e-2	2	321.146	2	6940.888	1
85		5	max	0	15	.898	3	.024	1	9.313e-3	3	NC	5	NC	2
86			min	0	1	-1.256	2	-.004	10	-1.34e-2	2	266.695	2	5879.688	1
87		6	max	0	15	.951	3	.023	1	1.01e-2	3	NC	15	NC	2
88			min	0	1	-1.313	2	-.006	10	-1.445e-2	2	241.839	2	6222.13	1
89		7	max	0	15	.983	3	.023	3	1.088e-2	3	NC	15	NC	2
90			min	0	1	-1.337	2	-.008	10	-1.551e-2	2	233.046	2	8521.502	1
91		8	max	0	15	.997	3	.025	3	1.166e-2	3	NC	15	NC	1
92			min	0	1	-1.334	2	-.012	2	-1.656e-2	2	234.059	2	9582.175	3
93		9	max	0	15	.999	3	.026	3	1.245e-2	3	NC	15	NC	1
94			min	0	1	-1.318	2	-.017	2	-1.762e-2	2	239.939	2	9033.279	3
95		10	max	0	1	.998	3	.026	3	1.323e-2	3	NC	15	NC	1
96			min	0	1	-1.308	2	-.019	2	-1.867e-2	2	243.954	2	8873.908	3
97		11	max	0	1	.999	3	.026	3	1.245e-2	3	NC	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.318	2	-.017	2	-1.762e-2	2	239.939	2	9033.279	3
99		12	max	0	1	.997	3	.025	3	1.166e-2	3	NC	15	NC	1
100			min	0	15	-1.334	2	-.012	2	-1.656e-2	2	234.059	2	9582.175	3
101		13	max	0	1	.983	3	.023	3	1.088e-2	3	NC	15	NC	2
102			min	0	15	-1.337	2	-.008	10	-1.551e-2	2	233.046	2	8521.502	1
103		14	max	0	1	.951	3	.023	1	1.01e-2	3	NC	15	NC	2
104			min	0	15	-1.313	2	-.006	10	-1.445e-2	2	241.839	2	6222.13	1
105		15	max	0	1	.898	3	.024	1	9.313e-3	3	NC	5	NC	2
106			min	0	15	-1.256	2	-.004	10	-1.34e-2	2	266.695	2	5879.688	1
107		16	max	0	1	.824	3	.02	1	8.53e-3	3	NC	5	NC	2
108			min	0	15	-1.16	2	-.004	10	-1.235e-2	2	321.146	2	6940.888	1
109		17	max	0	1	.73	3	.013	1	7.747e-3	3	NC	5	NC	1
110			min	0	15	-1.029	2	-.004	10	-1.129e-2	2	446.346	2	NC	1
111		18	max	0	1	.62	3	.01	3	6.963e-3	3	NC	5	NC	1
112			min	0	15	-.869	2	-.005	2	-1.024e-2	2	850.523	2	NC	1
113		19	max	0	1	.502	3	.009	3	6.18e-3	3	NC	1	NC	1
114			min	0	15	-.693	2	-.006	2	-9.183e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.215	2	.008	3	1.225e-2	3	NC	1	NC	1
116			min	0	1	-.182	3	-.006	2	-1.403e-2	2	NC	1	NC	1
117		2	max	0	15	.147	2	.01	3	1.289e-2	3	NC	4	NC	1
118			min	0	1	-.162	3	-.003	10	-1.416e-2	2	2204.582	2	NC	1
119		3	max	0	15	.097	1	.018	1	1.354e-2	3	NC	4	NC	2
120			min	0	1	-.148	3	-.003	10	-1.429e-2	2	1228.504	2	7917.714	1
121		4	max	0	15	.076	1	.026	1	1.418e-2	3	NC	4	NC	2
122			min	0	1	-.144	3	-.003	10	-1.442e-2	2	981.445	2	5483.64	1
123		5	max	0	15	.076	1	.03	1	1.483e-2	3	NC	4	NC	2
124			min	0	1	-.154	3	-.003	10	-1.456e-2	2	961.755	2	4848.823	1
125		6	max	0	15	.096	1	.027	1	1.547e-2	3	NC	3	NC	2
126			min	0	1	-.175	3	-.005	10	-1.469e-2	2	1134.11	2	5245.602	1
127		7	max	0	15	.132	1	.021	3	1.612e-2	3	NC	4	NC	2
128			min	0	1	-.205	3	-.007	10	-1.482e-2	2	1713.05	2	7198.41	1
129		8	max	0	15	.182	2	.022	3	1.676e-2	3	NC	1	NC	1
130			min	0	1	-.237	3	-.01	2	-1.495e-2	2	2711.037	3	NC	1
131		9	max	0	15	.231	2	.022	3	1.741e-2	3	NC	4	NC	1
132			min	0	1	-.265	3	-.015	2	-1.508e-2	2	1805.36	3	NC	1
133		10	max	0	1	.253	2	.023	3	1.805e-2	3	NC	4	NC	1
134			min	0	1	-.277	3	-.017	2	-1.521e-2	2	1575.066	3	NC	1
135		11	max	0	1	.231	2	.022	3	1.741e-2	3	NC	4	NC	1
136			min	0	15	-.265	3	-.015	2	-1.508e-2	2	1805.36	3	NC	1
137		12	max	0	1	.182	2	.022	3	1.676e-2	3	NC	1	NC	1
138			min	0	15	-.237	3	-.01	2	-1.495e-2	2	2711.037	3	NC	1
139		13	max	0	1	.132	1	.021	3	1.612e-2	3	NC	4	NC	2
140			min	0	15	-.205	3	-.007	10	-1.482e-2	2	1713.05	2	7198.41	1
141		14	max	0	1	.096	1	.027	1	1.547e-2	3	NC	3	NC	2
142			min	0	15	-.175	3	-.005	10	-1.469e-2	2	1134.11	2	5245.602	1
143		15	max	0	1	.076	1	.03	1	1.483e-2	3	NC	4	NC	2
144			min	0	15	-.154	3	-.003	10	-1.456e-2	2	961.755	2	4848.823	1
145		16	max	0	1	.076	1	.026	1	1.418e-2	3	NC	4	NC	2
146			min	0	15	-.144	3	-.003	10	-1.442e-2	2	981.445	2	5483.64	1
147		17	max	0	1	.097	1	.018	1	1.354e-2	3	NC	4	NC	2
148			min	0	15	-.148	3	-.003	10	-1.429e-2	2	1228.504	2	7917.714	1
149		18	max	0	1	.147	2	.01	3	1.289e-2	3	NC	4	NC	1
150			min	0	15	-.162	3	-.003	10	-1.416e-2	2	2204.582	2	NC	1
151		19	max	0	1	.215	2	.008	3	1.225e-2	3	NC	1	NC	1
152			min	0	15	-.182	3	-.006	2	-1.403e-2	2	NC	1	NC	1
153	M2	1	max	.007	2	.011	2	.005	1	-4.106e-6	10	NC	1	NC	1
154			min	-.01	3	-.016	3	0	15	-9.9e-5	1	6435.488	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.007	2	.009	2	.004	1	-3.866e-6	10	NC	1	NC	1
156		min	-.01	3	-.016	3	0	15	-9.357e-5	1	7378.064	2	NC	1
157	3	max	.007	2	.008	2	.004	1	-3.627e-6	10	NC	1	NC	1
158		min	-.009	3	-.015	3	0	15	-8.814e-5	1	8627.563	2	NC	1
159	4	max	.006	2	.007	2	.004	1	-3.388e-6	10	NC	1	NC	1
160		min	-.009	3	-.015	3	0	15	-8.271e-5	1	NC	1	NC	1
161	5	max	.006	2	.005	2	.003	1	-3.149e-6	10	NC	1	NC	1
162		min	-.008	3	-.014	3	0	15	-7.728e-5	1	NC	1	NC	1
163	6	max	.005	2	.004	2	.003	1	-2.909e-6	10	NC	1	NC	1
164		min	-.007	3	-.013	3	0	15	-7.186e-5	1	NC	1	NC	1
165	7	max	.005	2	.003	2	.002	1	-2.67e-6	10	NC	1	NC	1
166		min	-.007	3	-.013	3	0	15	-6.643e-5	1	NC	1	NC	1
167	8	max	.005	2	.002	2	.002	1	-2.431e-6	10	NC	1	NC	1
168		min	-.006	3	-.012	3	0	15	-6.1e-5	1	NC	1	NC	1
169	9	max	.004	2	.001	2	.002	1	-2.191e-6	10	NC	1	NC	1
170		min	-.006	3	-.011	3	0	15	-5.557e-5	1	NC	1	NC	1
171	10	max	.004	2	0	2	.001	1	-1.952e-6	10	NC	1	NC	1
172		min	-.005	3	-.01	3	0	15	-5.014e-5	1	NC	1	NC	1
173	11	max	.003	2	0	2	.001	1	-1.713e-6	10	NC	1	NC	1
174		min	-.005	3	-.009	3	0	15	-4.471e-5	1	NC	1	NC	1
175	12	max	.003	2	-.001	2	0	1	-1.473e-6	10	NC	1	NC	1
176		min	-.004	3	-.008	3	0	15	-3.929e-5	1	NC	1	NC	1
177	13	max	.002	2	-.001	15	0	1	-1.234e-6	10	NC	1	NC	1
178		min	-.003	3	-.008	3	0	15	-3.386e-5	1	NC	1	NC	1
179	14	max	.002	2	-.001	15	0	1	-9.948e-7	10	NC	1	NC	1
180		min	-.003	3	-.006	3	0	15	-2.843e-5	1	NC	1	NC	1
181	15	max	.002	2	-.001	15	0	1	-7.555e-7	10	NC	1	NC	1
182		min	-.002	3	-.005	3	0	15	-2.3e-5	1	NC	1	NC	1
183	16	max	.001	2	0	15	0	1	-5.162e-7	10	NC	1	NC	1
184		min	-.002	3	-.004	3	0	15	-1.757e-5	1	NC	1	NC	1
185	17	max	0	2	0	15	0	1	-2.768e-7	10	NC	1	NC	1
186		min	-.001	3	-.003	3	0	15	-1.214e-5	1	NC	1	NC	1
187	18	max	0	2	0	15	0	1	-3.753e-8	10	NC	1	NC	1
188		min	0	3	-.001	4	0	15	-6.715e-6	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	2.018e-7	10	NC	1	NC	1
190		min	0	1	0	1	0	1	-1.287e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1.477e-7	3	NC	1	NC	1
192		min	0	1	0	1	0	1	-3.718e-7	1	NC	1	NC	1
193	2	max	0	3	0	15	0	10	1.239e-5	1	NC	1	NC	1
194		min	0	2	-.003	4	0	3	5.209e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	2	2.515e-5	1	NC	1	NC	1
196		min	0	2	-.006	4	0	3	1.054e-6	15	NC	1	NC	1
197	4	max	.001	3	-.002	15	0	1	3.791e-5	1	NC	1	NC	1
198		min	-.001	2	-.009	4	0	3	1.588e-6	15	NC	1	NC	1
199	5	max	.002	3	-.003	15	0	1	5.067e-5	1	NC	1	NC	1
200		min	-.002	2	-.012	4	0	3	2.121e-6	15	8807.766	4	NC	1
201	6	max	.002	3	-.003	15	0	1	6.343e-5	1	NC	2	NC	1
202		min	-.002	2	-.015	4	0	3	2.654e-6	15	7109.275	4	NC	1
203	7	max	.003	3	-.004	15	0	1	7.619e-5	1	NC	5	NC	1
204		min	-.003	2	-.017	4	0	12	3.187e-6	15	6087.74	4	NC	1
205	8	max	.003	3	-.004	15	0	1	8.895e-5	1	NC	5	NC	1
206		min	-.003	2	-.019	4	0	15	3.721e-6	15	5457.351	4	NC	1
207	9	max	.004	3	-.005	15	0	1	1.017e-4	1	NC	5	NC	1
208		min	-.003	2	-.02	4	0	15	4.254e-6	15	5083.683	4	NC	1
209	10	max	.004	3	-.005	15	0	1	1.145e-4	1	NC	5	NC	1
210		min	-.004	2	-.021	4	0	15	4.787e-6	15	4901.393	4	NC	1
211	11	max	.005	3	-.005	15	0	1	1.272e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.021	4	0	15	5.321e-6	15	4883.544	4	NC	1
213		max	.005	3	-.005	15	.001	1	1.4e-4	1	NC	5	NC	1
214		min	-.005	2	-.021	4	0	15	5.854e-6	15	5031.257	4	NC	1
215		max	.006	3	-.005	15	.001	1	1.527e-4	1	NC	5	NC	1
216		min	-.005	2	-.019	4	0	15	6.387e-6	15	5375.458	4	NC	1
217		max	.006	3	-.004	15	.002	1	1.655e-4	1	NC	5	NC	1
218		min	-.005	2	-.017	4	0	15	6.921e-6	15	5993.003	4	NC	1
219		max	.007	3	-.003	15	.002	1	1.783e-4	1	NC	3	NC	1
220		min	-.006	2	-.015	4	0	15	7.454e-6	15	7054.6	4	NC	1
221		max	.007	3	-.003	15	.003	1	1.91e-4	1	NC	1	NC	1
222		min	-.006	2	-.012	4	0	15	7.987e-6	15	8973.962	4	NC	1
223		max	.008	3	-.002	15	.003	1	2.038e-4	1	NC	1	NC	1
224		min	-.007	2	-.008	4	0	15	8.521e-6	15	NC	1	NC	1
225		max	.008	3	-.001	15	.004	1	2.165e-4	1	NC	1	NC	1
226		min	-.007	2	-.005	4	0	15	9.054e-6	15	NC	1	NC	1
227		max	.009	3	0	10	.004	1	2.293e-4	1	NC	1	NC	1
228		min	-.008	2	-.002	3	0	15	9.587e-6	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	6.437e-5	1	NC	1	NC	2
230		min	0	3	-.009	3	-.004	1	2.719e-6	15	NC	1	5609.227	1
231		max	.002	1	.007	2	0	15	6.437e-5	1	NC	1	NC	2
232		min	0	3	-.009	3	-.004	1	2.719e-6	15	NC	1	6093.319	1
233		max	.002	1	.006	2	0	15	6.437e-5	1	NC	1	NC	2
234		min	0	3	-.008	3	-.004	1	2.719e-6	15	NC	1	6669.845	1
235		max	.002	1	.006	2	0	15	6.437e-5	1	NC	1	NC	2
236		min	0	3	-.008	3	-.003	1	2.719e-6	15	NC	1	7362.748	1
237		max	.002	1	.006	2	0	15	6.437e-5	1	NC	1	NC	2
238		min	0	3	-.007	3	-.003	1	2.719e-6	15	NC	1	8204.574	1
239		max	.002	1	.005	2	0	15	6.437e-5	1	NC	1	NC	2
240		min	0	3	-.007	3	-.003	1	2.719e-6	15	NC	1	9240.453	1
241		max	.002	1	.005	2	0	15	6.437e-5	1	NC	1	NC	1
242		min	0	3	-.006	3	-.002	1	2.719e-6	15	NC	1	NC	1
243		max	.001	1	.004	2	0	15	6.437e-5	1	NC	1	NC	1
244		min	0	3	-.006	3	-.002	1	2.719e-6	15	NC	1	NC	1
245		max	.001	1	.004	2	0	15	6.437e-5	1	NC	1	NC	1
246		min	0	3	-.005	3	-.002	1	2.719e-6	15	NC	1	NC	1
247		max	.001	1	.004	2	0	15	6.437e-5	1	NC	1	NC	1
248		min	0	3	-.005	3	-.001	1	2.719e-6	15	NC	1	NC	1
249		max	.001	1	.003	2	0	15	6.437e-5	1	NC	1	NC	1
250		min	0	3	-.004	3	-.001	1	2.719e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	6.437e-5	1	NC	1	NC	1
252		min	0	3	-.004	3	0	1	2.719e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	6.437e-5	1	NC	1	NC	1
254		min	0	3	-.003	3	0	1	2.719e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	6.437e-5	1	NC	1	NC	1
256		min	0	3	-.003	3	0	1	2.719e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	6.437e-5	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	2.719e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	6.437e-5	1	NC	1	NC	1
260		min	0	3	-.002	3	0	1	2.719e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.437e-5	1	NC	1	NC	1
262		min	0	3	-.001	3	0	1	2.719e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.437e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.719e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.437e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.719e-6	15	NC	1	NC	1
267	M6	max	.021	2	.033	2	0	1	0	1	NC	4	NC	1
268		min	-.031	3	-.047	3	0	1	0	1	1479.241	3	NC	1



Company : Schletter, Inc.  
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 Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.02	2	.03	2	0	1	0	1	NC	4	NC	1
270		min	-.029	3	-.044	3	0	1	0	1	1566.106	3	NC	1
271	3	max	.019	2	.027	2	0	1	0	1	NC	4	NC	1
272		min	-.028	3	-.042	3	0	1	0	1	1663.841	3	NC	1
273	4	max	.018	2	.024	2	0	1	0	1	NC	4	NC	1
274		min	-.026	3	-.039	3	0	1	0	1	1774.651	3	NC	1
275	5	max	.017	2	.022	2	0	1	0	1	NC	4	NC	1
276		min	-.024	3	-.036	3	0	1	0	1	1901.371	3	NC	1
277	6	max	.015	2	.019	2	0	1	0	1	NC	4	NC	1
278		min	-.022	3	-.034	3	0	1	0	1	2047.707	3	NC	1
279	7	max	.014	2	.016	2	0	1	0	1	NC	4	NC	1
280		min	-.021	3	-.031	3	0	1	0	1	2218.604	3	NC	1
281	8	max	.013	2	.014	2	0	1	0	1	NC	1	NC	1
282		min	-.019	3	-.029	3	0	1	0	1	2420.802	3	NC	1
283	9	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
284		min	-.017	3	-.026	3	0	1	0	1	2663.736	3	NC	1
285	10	max	.011	2	.009	2	0	1	0	1	NC	1	NC	1
286		min	-.016	3	-.023	3	0	1	0	1	2961.04	3	NC	1
287	11	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
288		min	-.014	3	-.021	3	0	1	0	1	3333.16	3	NC	1
289	12	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.012	3	-.018	3	0	1	0	1	3812.229	3	NC	1
291	13	max	.007	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.01	3	-.016	3	0	1	0	1	4451.804	3	NC	1
293	14	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.009	3	-.013	3	0	1	0	1	5348.287	3	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.007	3	-.01	3	0	1	0	1	6694.486	3	NC	1
297	16	max	.004	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.008	3	0	1	0	1	8940.288	3	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.011	3	0	1	0	1	NC	1	NC	1
313	5	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.014	3	0	1	0	1	8077.391	3	NC	1
315	6	max	.007	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.006	2	-.016	3	0	1	0	1	6811.808	3	NC	1
317	7	max	.008	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.019	3	0	1	0	1	6051.562	3	NC	1
319	8	max	.01	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.009	2	-.02	3	0	1	0	1	5516.753	4	NC	1
321	9	max	.011	3	-.005	15	0	1	0	1	NC	2	NC	1
322		min	-.01	2	-.021	3	0	1	0	1	5135.353	4	NC	1
323	10	max	.013	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.012	2	-.022	3	0	1	0	1	4948.26	4	NC	1
325	11	max	.014	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.022	3	0	1	0	1	4927.772	4	NC	1
327		12	max	.015	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.014	2	-.021	3	0	1	0	1	5074.684	4	NC	1
329		13	max	.017	3	-.005	15	0	1	0	1	NC	5	NC	1
330			min	-.015	2	-.02	3	0	1	0	1	5419.941	4	NC	1
331		14	max	.018	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.017	2	-.018	3	0	1	0	1	6040.825	4	NC	1
333		15	max	.02	3	-.004	15	0	1	0	1	NC	1	NC	1
334			min	-.018	2	-.016	3	0	1	0	1	7109.195	4	NC	1
335		16	max	.021	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.019	2	-.014	3	0	1	0	1	9041.712	4	NC	1
337		17	max	.022	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.021	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.024	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.022	2	-.008	3	0	1	0	1	NC	1	NC	1
341		19	max	.025	3	0	10	0	1	0	1	NC	1	NC	1
342			min	-.023	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	2	.022	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.026	3	0	1	0	1	NC	1	NC	1
345		2	max	.006	2	.021	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.024	3	0	1	0	1	NC	1	NC	1
347		3	max	.006	2	.02	2	0	1	0	1	NC	1	NC	1
348			min	-.001	3	-.023	3	0	1	0	1	NC	1	NC	1
349		4	max	.005	2	.019	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.021	3	0	1	0	1	NC	1	NC	1
351		5	max	.005	2	.017	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.02	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	2	.016	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
355		7	max	.004	2	.015	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.017	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	2	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	2	.012	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	2	.011	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	2	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	2	.007	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.001	2	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.011	2	0	15	9.9e-5	1	NC	1	NC	1
382			min	-.01	3	-.016	3	-.005	1	4.106e-6	10	6435.488	2	NC	1





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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.007	2	.009	2	0	15	9.357e-5	1	NC	1	NC	1
384			min	-.01	3	-.016	3	-.004	1	3.866e-6	10	7378.064	2	NC	1
385		3	max	.007	2	.008	2	0	15	8.814e-5	1	NC	1	NC	1
386			min	-.009	3	-.015	3	-.004	1	3.627e-6	10	8627.563	2	NC	1
387		4	max	.006	2	.007	2	0	15	8.271e-5	1	NC	1	NC	1
388			min	-.009	3	-.015	3	-.004	1	3.388e-6	10	NC	1	NC	1
389		5	max	.006	2	.005	2	0	15	7.728e-5	1	NC	1	NC	1
390			min	-.008	3	-.014	3	-.003	1	3.149e-6	10	NC	1	NC	1
391		6	max	.005	2	.004	2	0	15	7.186e-5	1	NC	1	NC	1
392			min	-.007	3	-.013	3	-.003	1	2.909e-6	10	NC	1	NC	1
393		7	max	.005	2	.003	2	0	15	6.643e-5	1	NC	1	NC	1
394			min	-.007	3	-.013	3	-.002	1	2.67e-6	10	NC	1	NC	1
395		8	max	.005	2	.002	2	0	15	6.1e-5	1	NC	1	NC	1
396			min	-.006	3	-.012	3	-.002	1	2.431e-6	10	NC	1	NC	1
397		9	max	.004	2	.001	2	0	15	5.557e-5	1	NC	1	NC	1
398			min	-.006	3	-.011	3	-.002	1	2.191e-6	10	NC	1	NC	1
399		10	max	.004	2	0	2	0	15	5.014e-5	1	NC	1	NC	1
400			min	-.005	3	-.01	3	-.001	1	1.952e-6	10	NC	1	NC	1
401		11	max	.003	2	0	2	0	15	4.471e-5	1	NC	1	NC	1
402			min	-.005	3	-.009	3	-.001	1	1.713e-6	10	NC	1	NC	1
403		12	max	.003	2	-.001	2	0	15	3.929e-5	1	NC	1	NC	1
404			min	-.004	3	-.008	3	0	1	1.473e-6	10	NC	1	NC	1
405		13	max	.002	2	-.001	15	0	15	3.386e-5	1	NC	1	NC	1
406			min	-.003	3	-.008	3	0	1	1.234e-6	10	NC	1	NC	1
407		14	max	.002	2	-.001	15	0	15	2.843e-5	1	NC	1	NC	1
408			min	-.003	3	-.006	3	0	1	9.948e-7	10	NC	1	NC	1
409		15	max	.002	2	-.001	15	0	15	2.3e-5	1	NC	1	NC	1
410			min	-.002	3	-.005	3	0	1	7.555e-7	10	NC	1	NC	1
411		16	max	.001	2	0	15	0	15	1.757e-5	1	NC	1	NC	1
412			min	-.002	3	-.004	3	0	1	5.162e-7	10	NC	1	NC	1
413		17	max	0	2	0	15	0	15	1.214e-5	1	NC	1	NC	1
414			min	-.001	3	-.003	3	0	1	2.768e-7	10	NC	1	NC	1
415		18	max	0	2	0	15	0	15	6.715e-6	1	NC	1	NC	1
416			min	0	3	-.001	4	0	1	3.753e-8	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	1.287e-6	1	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.018e-7	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	3.718e-7	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-1.477e-7	3	NC	1	NC	1
421		2	max	0	3	0	15	0	3	-5.209e-7	15	NC	1	NC	1
422			min	0	2	-.003	4	0	10	-1.239e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	0	3	-1.054e-6	15	NC	1	NC	1
424			min	0	2	-.006	4	0	2	-2.515e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	0	3	-1.588e-6	15	NC	1	NC	1
426			min	-.001	2	-.009	4	0	1	-3.791e-5	1	NC	1	NC	1
427		5	max	.002	3	-.003	15	0	3	-2.121e-6	15	NC	1	NC	1
428			min	-.002	2	-.012	4	0	1	-5.067e-5	1	8807.766	4	NC	1
429		6	max	.002	3	-.003	15	0	3	-2.654e-6	15	NC	2	NC	1
430			min	-.002	2	-.015	4	0	1	-6.343e-5	1	7109.275	4	NC	1
431		7	max	.003	3	-.004	15	0	12	-3.187e-6	15	NC	5	NC	1
432			min	-.003	2	-.017	4	0	1	-7.619e-5	1	6087.74	4	NC	1
433		8	max	.003	3	-.004	15	0	15	-3.721e-6	15	NC	5	NC	1
434			min	-.003	2	-.019	4	0	1	-8.895e-5	1	5457.351	4	NC	1
435		9	max	.004	3	-.005	15	0	15	-4.254e-6	15	NC	5	NC	1
436			min	-.003	2	-.02	4	0	1	-1.017e-4	1	5083.683	4	NC	1
437		10	max	.004	3	-.005	15	0	15	-4.787e-6	15	NC	5	NC	1
438			min	-.004	2	-.021	4	0	1	-1.145e-4	1	4901.393	4	NC	1
439		11	max	.005	3	-.005	15	0	15	-5.321e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.004	2	-.021	4	0	1	-1.272e-4	1	4883.544	4	NC	1
441		max	-.005	3	-.005	15	0	15	-5.854e-6	15	NC	5	NC	1
442		min	-.005	2	-.021	4	-.001	1	-1.4e-4	1	5031.257	4	NC	1
443		max	.006	3	-.005	15	0	15	-6.387e-6	15	NC	5	NC	1
444		min	-.005	2	-.019	4	-.001	1	-1.527e-4	1	5375.458	4	NC	1
445		max	.006	3	-.004	15	0	15	-6.921e-6	15	NC	5	NC	1
446		min	-.005	2	-.017	4	-.002	1	-1.655e-4	1	5993.003	4	NC	1
447		max	.007	3	-.003	15	0	15	-7.454e-6	15	NC	3	NC	1
448		min	-.006	2	-.015	4	-.002	1	-1.783e-4	1	7054.6	4	NC	1
449		max	.007	3	-.003	15	0	15	-7.987e-6	15	NC	1	NC	1
450		min	-.006	2	-.012	4	-.003	1	-1.91e-4	1	8973.962	4	NC	1
451		max	.008	3	-.002	15	0	15	-8.521e-6	15	NC	1	NC	1
452		min	-.007	2	-.008	4	-.003	1	-2.038e-4	1	NC	1	NC	1
453		max	.008	3	-.001	15	0	15	-9.054e-6	15	NC	1	NC	1
454		min	-.007	2	-.005	4	-.004	1	-2.165e-4	1	NC	1	NC	1
455		max	.009	3	0	10	0	15	-9.587e-6	15	NC	1	NC	1
456		min	-.008	2	-.002	3	-.004	1	-2.293e-4	1	NC	1	NC	1
457	M12	max	.002	1	.007	2	.004	1	-2.719e-6	15	NC	1	NC	2
458		min	0	3	-.009	3	0	15	-6.437e-5	1	NC	1	5609.227	1
459		max	.002	1	.007	2	.004	1	-2.719e-6	15	NC	1	NC	2
460		min	0	3	-.009	3	0	15	-6.437e-5	1	NC	1	6093.319	1
461		max	.002	1	.006	2	.004	1	-2.719e-6	15	NC	1	NC	2
462		min	0	3	-.008	3	0	15	-6.437e-5	1	NC	1	6669.845	1
463		max	.002	1	.006	2	.003	1	-2.719e-6	15	NC	1	NC	2
464		min	0	3	-.008	3	0	15	-6.437e-5	1	NC	1	7362.748	1
465		max	.002	1	.006	2	.003	1	-2.719e-6	15	NC	1	NC	2
466		min	0	3	-.007	3	0	15	-6.437e-5	1	NC	1	8204.574	1
467		max	.002	1	.005	2	.003	1	-2.719e-6	15	NC	1	NC	2
468		min	0	3	-.007	3	0	15	-6.437e-5	1	NC	1	9240.453	1
469		max	.002	1	.005	2	.002	1	-2.719e-6	15	NC	1	NC	1
470		min	0	3	-.006	3	0	15	-6.437e-5	1	NC	1	NC	1
471		max	.001	1	.004	2	.002	1	-2.719e-6	15	NC	1	NC	1
472		min	0	3	-.006	3	0	15	-6.437e-5	1	NC	1	NC	1
473		max	.001	1	.004	2	.002	1	-2.719e-6	15	NC	1	NC	1
474		min	0	3	-.005	3	0	15	-6.437e-5	1	NC	1	NC	1
475		max	.001	1	.004	2	.001	1	-2.719e-6	15	NC	1	NC	1
476		min	0	3	-.005	3	0	15	-6.437e-5	1	NC	1	NC	1
477		max	.001	1	.003	2	.001	1	-2.719e-6	15	NC	1	NC	1
478		min	0	3	-.004	3	0	15	-6.437e-5	1	NC	1	NC	1
479		max	0	1	.003	2	0	1	-2.719e-6	15	NC	1	NC	1
480		min	0	3	-.004	3	0	15	-6.437e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-2.719e-6	15	NC	1	NC	1
482		min	0	3	-.003	3	0	15	-6.437e-5	1	NC	1	NC	1
483		max	0	1	.002	2	0	1	-2.719e-6	15	NC	1	NC	1
484		min	0	3	-.003	3	0	15	-6.437e-5	1	NC	1	NC	1
485		max	0	1	.002	2	0	1	-2.719e-6	15	NC	1	NC	1
486		min	0	3	-.002	3	0	15	-6.437e-5	1	NC	1	NC	1
487		max	0	1	.001	2	0	1	-2.719e-6	15	NC	1	NC	1
488		min	0	3	-.002	3	0	15	-6.437e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.719e-6	15	NC	1	NC	1
490		min	0	3	-.001	3	0	15	-6.437e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.719e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.437e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.719e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.437e-5	1	NC	1	NC	1
495	M1	max	.011	3	.239	2	0	1	4.814e-3	2	NC	1	NC	1
496		min	-.007	2	-.074	3	0	15	-1.35e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.011	3	.116	2	0	15	2.365e-3	2	NC	5	NC	1
498			min	-.007	2	-.036	3	-.003	1	-6.707e-3	3	1106.999	2	NC	1
499		3	max	.011	3	.017	3	0	15	2.881e-5	10	NC	5	NC	1
500			min	-.007	2	-.013	2	-.005	1	-8.955e-5	3	537.52	2	NC	1
501		4	max	.011	3	.095	3	0	15	3.48e-3	2	NC	5	NC	1
502			min	-.007	2	-.155	2	-.004	1	-3.486e-3	3	343.483	2	NC	1
503		5	max	.011	3	.19	3	0	15	6.939e-3	2	NC	15	NC	1
504			min	-.007	2	-.302	2	-.003	1	-6.883e-3	3	250.407	2	NC	1
505		6	max	.011	3	.291	3	0	10	1.04e-2	2	9209.092	15	NC	1
506			min	-.007	2	-.442	2	-.001	1	-1.028e-2	3	198.758	2	NC	1
507		7	max	.01	3	.386	3	0	1	1.386e-2	2	7808.959	15	NC	1
508			min	-.007	2	-.567	2	0	3	-1.368e-2	3	168.092	2	NC	1
509		8	max	.01	3	.465	3	0	1	1.732e-2	2	6977.504	15	NC	1
510			min	-.007	2	-.665	2	0	15	-1.707e-2	3	149.88	2	NC	1
511		9	max	.01	3	.517	3	0	15	1.924e-2	2	6540.892	15	NC	1
512			min	-.006	2	-.727	2	0	1	-1.768e-2	3	140.364	2	NC	1
513		10	max	.01	3	.536	3	0	1	2.013e-2	2	6406.937	15	NC	1
514			min	-.006	2	-.748	2	0	10	-1.643e-2	3	137.576	2	NC	1
515		11	max	.01	3	.524	3	0	1	2.102e-2	2	6540.343	15	NC	1
516			min	-.006	2	-.726	2	0	15	-1.518e-2	3	140.85	2	NC	1
517		12	max	.009	3	.481	3	0	15	1.997e-2	2	6976.305	15	NC	1
518			min	-.006	2	-.662	2	0	1	-1.335e-2	3	151.272	2	NC	1
519		13	max	.009	3	.411	3	0	10	1.601e-2	2	7806.816	15	NC	1
520			min	-.006	2	-.56	2	0	1	-1.068e-2	3	171.315	2	NC	1
521		14	max	.009	3	.321	3	.001	1	1.204e-2	2	9205.429	15	NC	1
522			min	-.006	2	-.431	2	0	15	-8.014e-3	3	205.404	2	NC	1
523		15	max	.009	3	.218	3	.003	1	8.081e-3	2	NC	15	NC	1
524			min	-.006	2	-.288	2	0	15	-5.345e-3	3	263.646	2	NC	1
525		16	max	.008	3	.111	3	.004	1	4.119e-3	2	NC	5	NC	1
526			min	-.006	2	-.143	2	0	15	-2.675e-3	3	370.437	2	NC	1
527		17	max	.008	3	.006	3	.004	1	3.115e-4	1	NC	5	NC	1
528			min	-.006	2	-.007	2	0	15	-5.394e-6	3	596.281	2	NC	1
529		18	max	.008	3	.109	2	.003	1	4.268e-3	2	NC	5	NC	1
530			min	-.006	2	-.091	3	0	15	-1.281e-3	3	1253.473	2	NC	1
531		19	max	.008	3	.215	2	0	15	8.524e-3	2	NC	1	NC	1
532			min	-.006	2	-.182	3	0	1	-2.627e-3	3	NC	1	NC	1
533	M5	1	max	.032	3	.321	2	0	1	0	1	NC	1	NC	1
534			min	-.022	2	.006	15	0	1	0	1	NC	1	NC	1
535		2	max	.032	3	.156	2	0	1	0	1	NC	3	NC	1
536			min	-.023	2	.003	15	0	1	0	1	830.737	2	NC	1
537		3	max	.032	3	.047	3	0	1	0	1	NC	5	NC	1
538			min	-.023	2	-.036	2	0	1	0	1	383.815	2	NC	1
539		4	max	.031	3	.164	3	0	1	0	1	NC	15	NC	1
540			min	-.022	2	-.274	2	0	1	0	1	229.677	2	NC	1
541		5	max	.031	3	.34	3	0	1	0	1	8143.687	15	NC	1
542			min	-.022	2	-.539	2	0	1	0	1	158.666	2	NC	1
543		6	max	.03	3	.545	3	0	1	0	1	6212.973	15	NC	1
544			min	-.021	2	-.806	2	0	1	0	1	120.962	2	NC	1
545		7	max	.029	3	.748	3	0	1	0	1	5108.749	15	NC	1
546			min	-.021	2	-1.051	2	0	1	0	1	99.368	2	NC	1
547		8	max	.029	3	.921	3	0	1	0	1	4471.47	15	NC	1
548			min	-.021	2	-1.248	2	0	1	0	1	86.892	2	NC	1
549		9	max	.028	3	1.032	3	0	1	0	1	4146.004	15	NC	1
550			min	-.02	2	-1.374	2	0	1	0	1	80.516	2	NC	1
551		10	max	.027	3	1.073	3	0	1	0	1	4048.111	15	NC	1
552			min	-.02	2	-1.417	2	0	1	0	1	78.656	2	NC	1
553		11	max	.027	3	1.045	3	0	1	0	1	4146.444	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 24, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.019	2	-1.375	2	0	1	0	1	80.831	2	NC	1
555		12	max	.026	3	.953	3	0	1	0	1	4472.484	15	NC	1
556			min	-.019	2	-1.244	2	0	1	0	1	87.962	2	NC	1
557		13	max	.025	3	.804	3	0	1	0	1	5110.718	15	NC	1
558			min	-.019	2	-1.035	2	0	1	0	1	102.241	2	NC	1
559		14	max	.025	3	.617	3	0	1	0	1	6216.679	15	NC	1
560			min	-.018	2	-.778	2	0	1	0	1	127.671	2	NC	1
561		15	max	.024	3	.411	3	0	1	0	1	8150.847	15	NC	1
562			min	-.018	2	-.503	2	0	1	0	1	173.925	2	NC	1
563		16	max	.023	3	.204	3	0	1	0	1	NC	15	NC	1
564			min	-.018	2	-.239	2	0	1	0	1	266.116	2	NC	1
565		17	max	.023	3	.015	3	0	1	0	1	NC	5	NC	1
566			min	-.017	2	-.019	2	0	1	0	1	479.982	2	NC	1
567		18	max	.023	3	.136	2	0	1	0	1	NC	5	NC	1
568			min	-.017	2	-.141	3	0	1	0	1	1107.374	2	NC	1
569		19	max	.023	3	.253	2	0	1	0	1	NC	1	NC	1
570			min	-.017	2	-.277	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.011	3	.239	2	0	15	1.35e-2	3	NC	1	NC	1
572			min	-.007	2	-.074	3	0	1	-4.814e-3	2	NC	1	NC	1
573		2	max	.011	3	.116	2	.003	1	6.707e-3	3	NC	5	NC	1
574			min	-.007	2	-.036	3	0	15	-2.365e-3	2	1106.999	2	NC	1
575		3	max	.011	3	.017	3	.005	1	8.955e-5	3	NC	5	NC	1
576			min	-.007	2	-.013	2	0	15	-2.881e-5	10	537.52	2	NC	1
577		4	max	.011	3	.095	3	.004	1	3.486e-3	3	NC	5	NC	1
578			min	-.007	2	-.155	2	0	15	-3.48e-3	2	343.483	2	NC	1
579		5	max	.011	3	.19	3	.003	1	6.883e-3	3	NC	15	NC	1
580			min	-.007	2	-.302	2	0	15	-6.939e-3	2	250.407	2	NC	1
581		6	max	.011	3	.291	3	.001	1	1.028e-2	3	9209.092	15	NC	1
582			min	-.007	2	-.442	2	0	10	-1.04e-2	2	198.758	2	NC	1
583		7	max	.01	3	.386	3	0	3	1.368e-2	3	7808.959	15	NC	1
584			min	-.007	2	-.567	2	0	1	-1.386e-2	2	168.092	2	NC	1
585		8	max	.01	3	.465	3	0	15	1.707e-2	3	6977.504	15	NC	1
586			min	-.007	2	-.665	2	0	1	-1.732e-2	2	149.88	2	NC	1
587		9	max	.01	3	.517	3	0	1	1.768e-2	3	6540.892	15	NC	1
588			min	-.006	2	-.727	2	0	15	-1.924e-2	2	140.364	2	NC	1
589		10	max	.01	3	.536	3	0	10	1.643e-2	3	6406.937	15	NC	1
590			min	-.006	2	-.748	2	0	1	-2.013e-2	2	137.576	2	NC	1
591		11	max	.01	3	.524	3	0	15	1.518e-2	3	6540.343	15	NC	1
592			min	-.006	2	-.726	2	0	1	-2.102e-2	2	140.85	2	NC	1
593		12	max	.009	3	.481	3	0	1	1.335e-2	3	6976.305	15	NC	1
594			min	-.006	2	-.662	2	0	15	-1.997e-2	2	151.272	2	NC	1
595		13	max	.009	3	.411	3	0	1	1.068e-2	3	7806.816	15	NC	1
596			min	-.006	2	-.56	2	0	10	-1.601e-2	2	171.315	2	NC	1
597		14	max	.009	3	.321	3	0	15	8.014e-3	3	9205.429	15	NC	1
598			min	-.006	2	-.431	2	-.001	1	-1.204e-2	2	205.404	2	NC	1
599		15	max	.009	3	.218	3	0	15	5.345e-3	3	NC	15	NC	1
600			min	-.006	2	-.288	2	-.003	1	-8.081e-3	2	263.646	2	NC	1
601		16	max	.008	3	.111	3	0	15	2.675e-3	3	NC	5	NC	1
602			min	-.006	2	-.143	2	-.004	1	-4.119e-3	2	370.437	2	NC	1
603		17	max	.008	3	.006	3	0	15	5.394e-6	3	NC	5	NC	1
604			min	-.006	2	-.007	2	-.004	1	-3.115e-4	1	596.281	2	NC	1
605		18	max	.008	3	.109	2	0	15	1.281e-3	3	NC	5	NC	1
606			min	-.006	2	-.091	3	-.003	1	-4.268e-3	2	1253.473	2	NC	1
607		19	max	.008	3	.215	2	0	1	2.627e-3	3	NC	1	NC	1
608			min	-.006	2	-.182	3	0	15	-8.524e-3	2	NC	1	NC	1



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Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

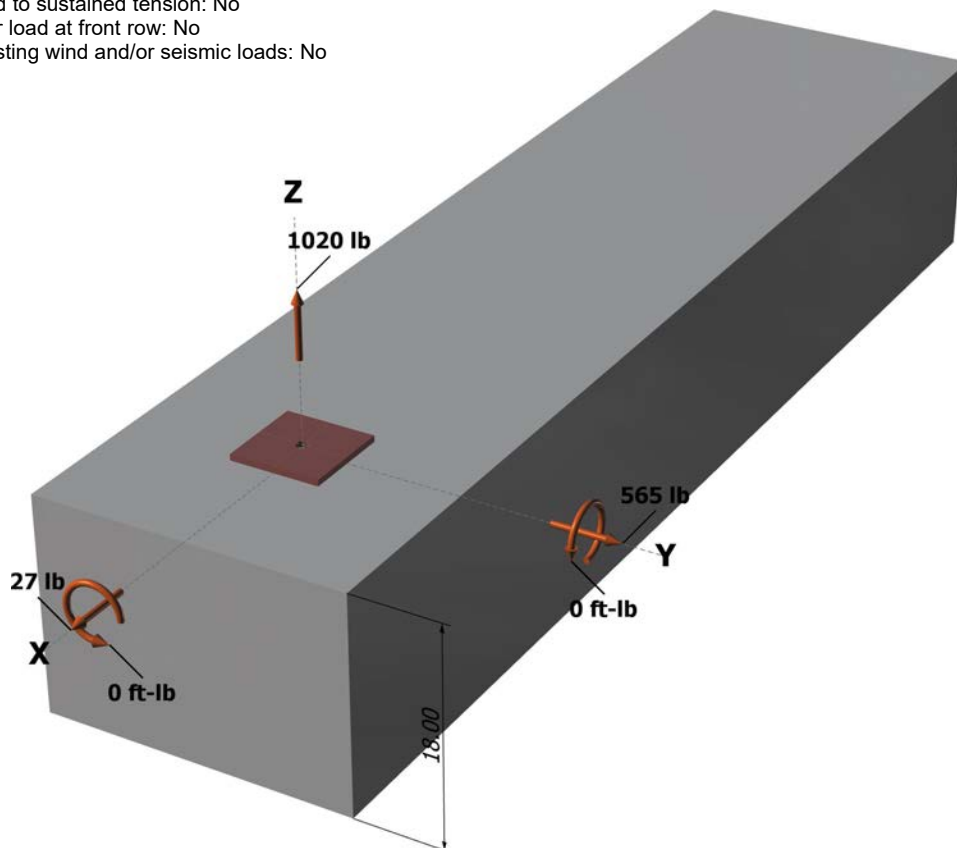
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

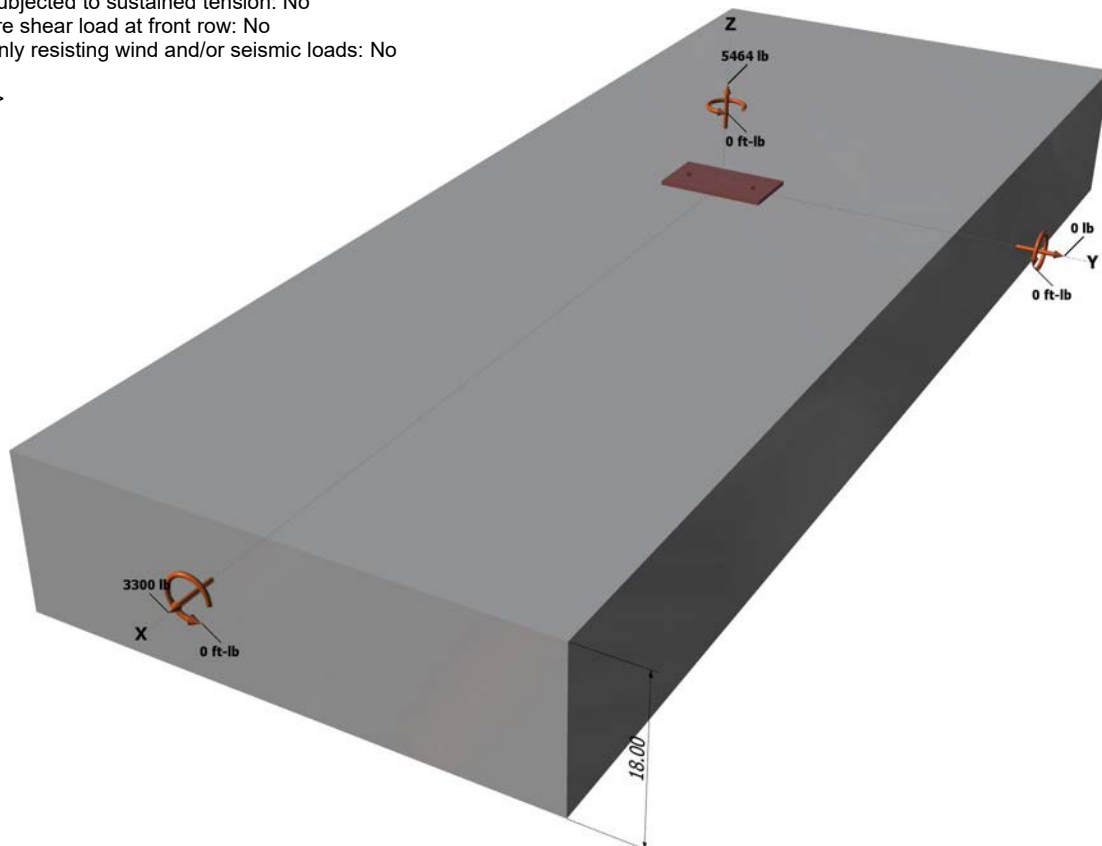
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



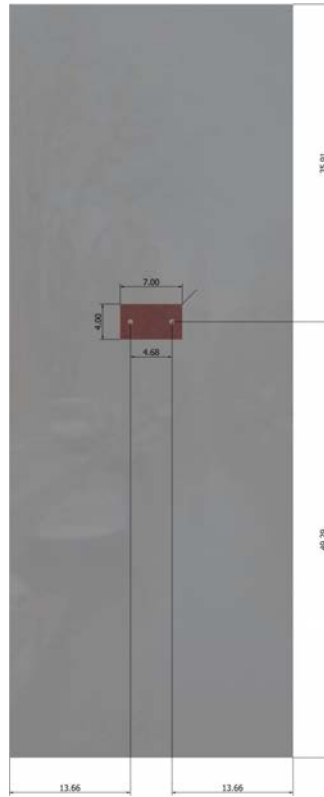
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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### 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

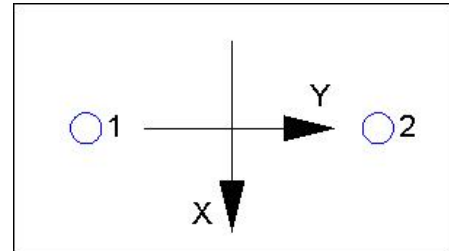
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



### 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

### 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

### 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.