

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

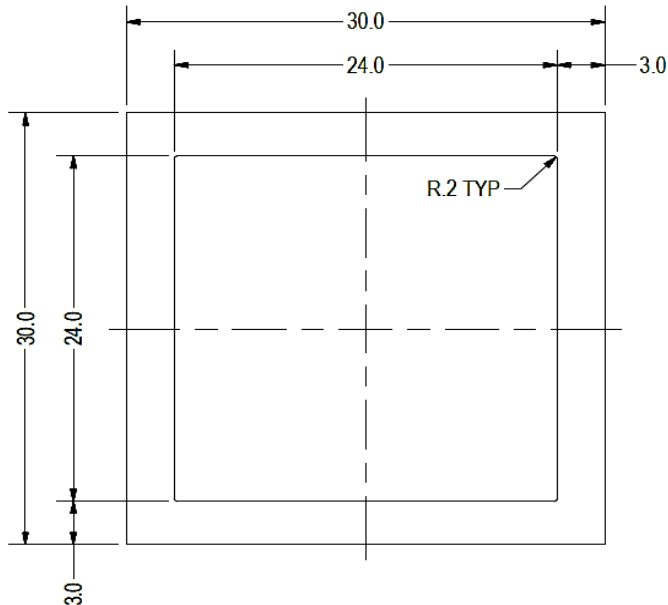
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.051 k-ft
P_n =	0.249 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	14%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.642 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	17%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.958 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	19%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.243 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	15%



A cross brace kit is required every 13 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

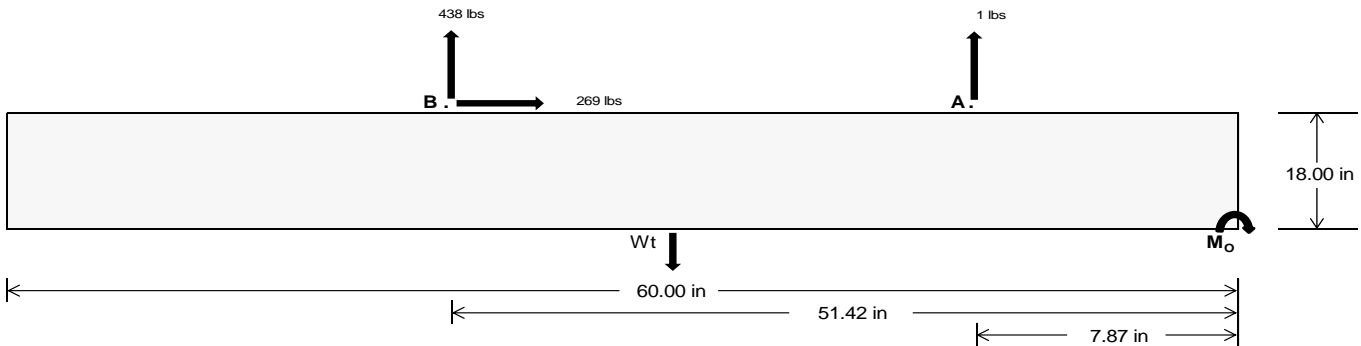
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	10.88	1825.50	k
Compressive Load =	1484.39	1360.88	k
Lateral Load =	41.46	1119.82	k
Moment (Weak Axis) =	0.07	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27377.4$ in-lbs
Resisting Force Required = 912.58 lbs
S.F. = 1.67
Weight Required = 1520.97 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 269.06 lbs
Friction = 0.4
Weight Required = 672.65 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 269.06 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

	Ballast Width			
	22 in	23 in	24 in	25 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$	1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	571 lbs	571 lbs	571 lbs	571 lbs	440 lbs	440 lbs	440 lbs	440 lbs	707 lbs	707 lbs	707 lbs	707 lbs	-3 lbs	-3 lbs	-3 lbs	-3 lbs
F_B	402 lbs	402 lbs	402 lbs	402 lbs	570 lbs	570 lbs	570 lbs	570 lbs	692 lbs	692 lbs	692 lbs	692 lbs	-876 lbs	-876 lbs	-876 lbs	-876 lbs
F_V	70 lbs	70 lbs	70 lbs	70 lbs	491 lbs	491 lbs	491 lbs	491 lbs	415 lbs	415 lbs	415 lbs	415 lbs	-538 lbs	-538 lbs	-538 lbs	-538 lbs
P_{total}	2967 lbs	3058 lbs	3148 lbs	3239 lbs	3004 lbs	3094 lbs	3185 lbs	3275 lbs	3393 lbs	3483 lbs	3574 lbs	3665 lbs	317 lbs	372 lbs	426 lbs	481 lbs
M	440 lbs-ft	440 lbs-ft	440 lbs-ft	440 lbs-ft	530 lbs-ft	530 lbs-ft	530 lbs-ft	530 lbs-ft	691 lbs-ft	691 lbs-ft	691 lbs-ft	691 lbs-ft	752 lbs-ft	752 lbs-ft	752 lbs-ft	752 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.14 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.20 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.37 ft	2.02 ft	1.76 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	266.1 psf	263.9 psf	262.0 psf	260.2 psf	258.3 psf	256.5 psf	254.9 psf	253.4 psf	279.7 psf	277.0 psf	274.5 psf	272.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	381.3 psf	374.2 psf	367.6 psf	361.6 psf	397.0 psf	389.2 psf	382.1 psf	375.5 psf	460.5 psf	450.0 psf	440.3 psf	431.4 psf	870.8 psf	270.1 psf	192.9 psf	164.3 psf

Maximum Bearing Pressure = 871 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

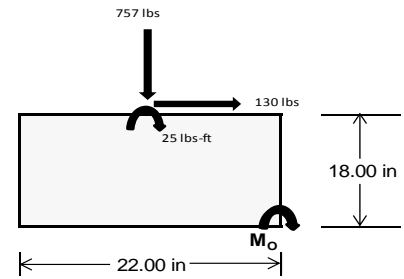
Overturning Check

$M_o = 473.7 \text{ ft-lbs}$
 Resisting Force Required = 516.77 lbs
 S.F. = 1.67
 Weight Required = 861.28 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	144 lbs	154 lbs	85 lbs	327 lbs	757 lbs	281 lbs	84 lbs	0 lbs	28 lbs
F_v	22 lbs	172 lbs	23 lbs	15 lbs	130 lbs	18 lbs	22 lbs	172 lbs	23 lbs
P_{total}	2612 lbs	2622 lbs	2553 lbs	2677 lbs	3107 lbs	2631 lbs	806 lbs	721 lbs	750 lbs
M	63 lbs-ft	292 lbs-ft	68 lbs-ft	41 lbs-ft	220 lbs-ft	54 lbs-ft	64 lbs-ft	291 lbs-ft	68 lbs-ft
e	0.02 ft	0.11 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.08 ft	0.40 ft	0.09 ft
$L/6$	0.31 ft	1.61 ft	1.78 ft	1.80 ft	1.69 ft	1.79 ft	1.68 ft	1.03 ft	1.65 ft
f_{min}	262.5 sqft	181.9 sqft	254.2 sqft	277.5 sqft	260.3 sqft	267.9 sqft	65.2 sqft	-25.4 sqft	57.6 sqft
f_{max}	307.4 psf	390.3 psf	302.8 psf	306.6 psf	417.5 psf	306.1 psf	110.6 psf	182.8 psf	106.0 psf



Maximum Bearing Pressure = 418 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

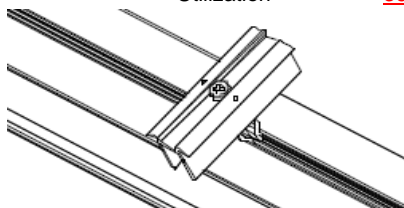
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

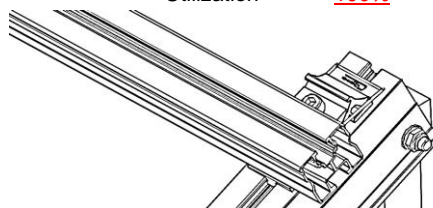
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.434 k
Allowable Uplift =	1.214 k
Utilization =	<u>36%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.114 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

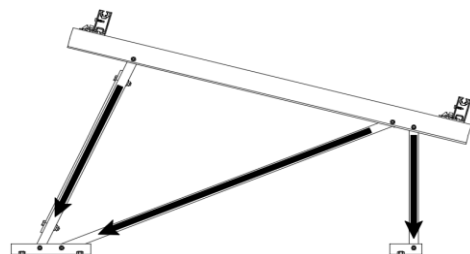
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.142 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Diagonal Strut

Maximum Axial Load =	0.642 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



Rear Strut

Maximum Axial Load =	1.197 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.243 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.108 in
	<u>0.108 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$226.543$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$235.251$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.211 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.24 \\ &22.039 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.24 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.6 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Load Combinations (Continued)

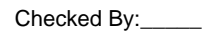
	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...
10	ASD 1.0D + 1.0W	Yes	Y			1	1			4	1										
11	ASD 1.0D + 0.75L + 0.75W + 0....	Yes	Y			1	1	3	.75	4	.75										
12	ASD 0.6D + 1.0W	Yes	Y			2	.6					5	1								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y			1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y			1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y			1	.362					6	.875								

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	212.697	2	305.012	2	-.009	10	0	4	0	1	0	1
2		min	-268.625	3	-426.418	3	-2.201	4	0	3	0	1	0	1
3	N7	max	.004	3	437.563	1	-.199	10	0	12	0	1	0	1
4		min	-.174	2	6.025	12	-31.659	4	-.051	4	0	1	0	1
5	N15	max	0	15	1141.838	1	.644	1	.001	1	0	1	0	1
6		min	-1.811	2	-8.369	3	-31.889	5	-.051	4	0	1	0	1
7	N16	max	810.506	2	1046.834	2	-.168	10	0	1	0	1	0	1
8		min	-861.398	3	-1404.229	3	-224.686	4	0	3	0	1	0	1
9	N23	max	.004	3	437.215	1	3.631	1	.006	1	0	1	0	1
10		min	-.174	2	6.387	12	-29.543	5	-.047	5	0	1	0	1
11	N24	max	213.186	2	309.341	2	47.702	3	.002	4	0	1	0	1
12		min	-268.756	3	-423.928	3	-3.66	5	0	3	0	1	0	1
13	Totals:	max	1234.228	2	3574.494	1	0	1						
14		min	-1398.786	3	-2246.526	3	-321.773	4						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	293.592	1	.652	6	1.375	4	0	12	0	3	0	1
2			min	-366.82	3	.153	15	-.039	3	-.001	1	0	1	0	1
3		2	max	293.718	1	.601	6	1.26	4	0	12	0	5	0	15
4			min	-366.725	3	.141	15	-.039	3	-.001	1	0	1	0	6
5		3	max	293.843	1	.549	6	1.146	4	0	12	0	5	0	15
6			min	-366.631	3	.129	15	-.039	3	-.001	1	0	1	0	6
7		4	max	293.969	1	.498	6	1.031	4	0	12	0	4	0	15
8			min	-366.537	3	.117	15	-.039	3	-.001	1	0	3	0	6
9		5	max	294.095	1	.447	6	.917	4	0	12	0	4	0	15
10			min	-366.442	3	.105	15	-.039	3	-.001	1	0	3	0	6
11		6	max	294.221	1	.396	6	.802	4	0	12	0	4	0	15
12			min	-366.348	3	.093	15	-.039	3	-.001	1	0	3	0	6
13		7	max	294.347	1	.345	6	.688	4	0	12	0	4	0	15
14			min	-366.253	3	.081	15	-.039	3	-.001	1	0	3	0	6
15		8	max	294.473	1	.294	6	.574	4	0	12	.001	4	0	15
16			min	-366.159	3	.069	15	-.039	3	-.001	1	0	3	0	6
17		9	max	294.599	1	.243	6	.558	1	0	12	.001	4	0	15
18			min	-366.065	3	.057	15	-.039	3	-.001	1	0	3	0	6
19		10	max	294.725	1	.191	6	.558	1	0	12	.001	4	0	15
20			min	-365.97	3	.044	15	-.039	3	-.001	1	0	3	0	6
21		11	max	294.85	1	.14	6	.558	1	0	12	.001	4	0	15
22			min	-365.876	3	.032	12	-.039	3	-.001	1	0	3	0	6
23		12	max	294.976	1	.1	2	.558	1	0	12	.001	4	0	15
24			min	-365.781	3	.012	12	-.039	3	-.001	1	0	3	0	6
25		13	max	295.102	1	.06	2	.558	1	0	12	.001	4	0	15
26			min	-365.687	3	-.014	3	-.141	5	-.001	1	0	3	0	6
27		14	max	295.228	1	.02	2	.558	1	0	12	.001	4	0	15
28			min	-365.593	3	-.044	3	-.255	5	-.001	1	0	3	0	6







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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	957.867	1	.053	2	.172	1	0	1	.001	4	0	12
144			min	-1195.233	3	-.154	3	-.411	5	0	5	0	3	0	2
145		16	max	957.993	1	.013	2	.172	1	0	1	.001	4	0	12
146			min	-1195.138	3	-.184	3	-.525	5	0	5	0	3	0	2
147		17	max	958.118	1	-.026	2	.172	1	0	1	.001	4	0	12
148			min	-1195.044	3	-.214	3	-.639	5	0	5	0	3	0	2
149		18	max	958.244	1	-.061	15	.172	1	0	1	0	4	0	3
150			min	-1194.949	3	-.244	3	-.754	5	0	5	0	3	0	2
151		19	max	958.37	1	-.073	15	.172	1	0	1	0	4	0	3
152			min	-1194.855	3	-.279	4	-.868	5	0	5	0	3	0	2
153	M7	1	max	641.788	2	1.774	4	.016	3	0	14	0	4	0	2
154			min	-551.446	3	.422	15	-1.324	5	0	3	0	3	0	3
155		2	max	641.719	2	1.597	4	.016	3	0	14	0	4	0	2
156			min	-551.498	3	.381	15	-1.191	5	0	3	0	3	0	3
157		3	max	641.65	2	1.42	4	.016	3	0	14	0	2	0	2
158			min	-551.55	3	.339	15	-1.057	5	0	3	0	3	0	3
159		4	max	641.58	2	1.243	4	.016	3	0	14	0	2	0	2
160			min	-551.602	3	.298	15	-.923	5	0	3	0	5	0	3
161		5	max	641.511	2	1.066	4	.016	3	0	14	0	2	0	15
162			min	-551.654	3	.256	15	-.79	5	0	3	0	5	0	3
163		6	max	641.442	2	.889	4	.016	3	0	14	0	2	0	15
164			min	-551.706	3	.214	15	-.656	5	0	3	0	5	0	3
165		7	max	641.372	2	.713	4	.016	3	0	14	0	2	0	15
166			min	-551.758	3	.173	15	-.522	5	0	3	0	5	0	6
167		8	max	641.303	2	.536	4	.016	3	0	14	0	2	0	15
168			min	-551.81	3	.131	15	-.388	5	0	3	0	5	-.001	6
169		9	max	641.234	2	.361	2	.016	3	0	14	0	2	0	15
170			min	-551.862	3	.066	12	-.255	5	0	3	0	5	-.001	6
171		10	max	641.164	2	.223	2	.016	3	0	14	0	2	0	15
172			min	-551.914	3	-.011	3	-.121	5	0	3	0	5	-.001	6
173		11	max	641.095	2	.085	2	.016	3	0	14	0	2	0	15
174			min	-551.966	3	-.115	3	-.005	10	0	3	0	5	-.001	6
175		12	max	641.026	2	-.035	15	.149	4	0	14	0	2	0	15
176			min	-552.018	3	-.218	3	-.005	10	0	3	0	5	-.001	6
177		13	max	640.956	2	-.077	15	.283	4	0	14	0	2	0	15
178			min	-552.07	3	-.349	6	-.005	10	0	3	0	5	-.001	6
179		14	max	640.887	2	-.118	15	.416	4	0	14	0	2	0	15
180			min	-552.122	3	-.526	6	-.005	10	0	3	0	5	-.001	6
181		15	max	640.818	2	-.16	15	.55	4	0	14	0	2	0	15
182			min	-552.174	3	-.703	6	-.005	10	0	3	0	5	0	6
183		16	max	640.748	2	-.201	15	.684	4	0	14	0	2	0	15
184			min	-552.226	3	-.879	6	-.005	10	0	3	0	5	0	6
185		17	max	640.679	2	-.243	15	.817	4	0	14	0	2	0	15
186			min	-552.278	3	-1.056	6	-.005	10	0	3	0	5	0	6
187		18	max	640.61	2	-.284	15	.951	4	0	14	0	2	0	15
188			min	-552.33	3	-1.233	6	-.005	10	0	3	0	5	0	6
189		19	max	640.541	2	-.326	15	1.085	4	0	14	0	14	0	1
190			min	-552.382	3	-1.41	6	-.005	10	0	3	0	3	0	1
191	M8	1	max	1140.673	1	0	1	.792	1	0	1	0	4	0	1
192			min	-9.242	3	0	1	-31.275	4	0	1	0	1	0	1
193		2	max	1140.738	1	0	1	.792	1	0	1	0	1	0	1
194			min	-9.194	3	0	1	-31.331	4	0	1	-.003	4	0	1
195		3	max	1140.802	1	0	1	.792	1	0	1	0	1	0	1
196			min	-9.145	3	0	1	-31.388	4	0	1	-.006	4	0	1
197		4	max	1140.867	1	0	1	.792	1	0	1	0	1	0	1
198			min	-9.097	3	0	1	-31.444	4	0	1	-.008	4	0	1
199		5	max	1140.932	1	0	1	.792	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-9.048	3	0	1	-31.5	4	0	1	-.011	4	0	1
201		6	max	1140.996	1	0	1	.792	1	0	1	0	1	0	1
202			min	-8.999	3	0	1	-31.556	4	0	1	-.014	4	0	1
203		7	max	1141.061	1	0	1	.792	1	0	1	0	1	0	1
204			min	-8.951	3	0	1	-31.612	4	0	1	-.017	4	0	1
205		8	max	1141.126	1	0	1	.792	1	0	1	0	1	0	1
206			min	-8.902	3	0	1	-31.668	4	0	1	-.02	4	0	1
207		9	max	1141.19	1	0	1	.792	1	0	1	0	1	0	1
208			min	-8.854	3	0	1	-31.724	4	0	1	-.023	4	0	1
209		10	max	1141.255	1	0	1	.792	1	0	1	0	1	0	1
210			min	-8.805	3	0	1	-31.78	4	0	1	-.025	4	0	1
211		11	max	1141.32	1	0	1	.792	1	0	1	0	1	0	1
212			min	-8.757	3	0	1	-31.836	4	0	1	-.028	4	0	1
213		12	max	1141.385	1	0	1	.792	1	0	1	0	1	0	1
214			min	-8.708	3	0	1	-31.892	4	0	1	-.031	4	0	1
215		13	max	1141.449	1	0	1	.792	1	0	1	0	1	0	1
216			min	-8.66	3	0	1	-31.948	4	0	1	-.034	4	0	1
217		14	max	1141.514	1	0	1	.792	1	0	1	0	1	0	1
218			min	-8.611	3	0	1	-32.004	4	0	1	-.037	4	0	1
219		15	max	1141.579	1	0	1	.792	1	0	1	0	1	0	1
220			min	-8.563	3	0	1	-32.06	4	0	1	-.04	4	0	1
221		16	max	1141.643	1	0	1	.792	1	0	1	.001	1	0	1
222			min	-8.514	3	0	1	-32.117	4	0	1	-.042	4	0	1
223		17	max	1141.708	1	0	1	.792	1	0	1	.001	1	0	1
224			min	-8.466	3	0	1	-32.173	4	0	1	-.045	4	0	1
225		18	max	1141.773	1	0	1	.792	1	0	1	.001	1	0	1
226			min	-8.417	3	0	1	-32.229	4	0	1	-.048	4	0	1
227		19	max	1141.838	1	0	1	.792	1	0	1	.001	1	0	1
228			min	-8.369	3	0	1	-32.285	4	0	1	-.051	4	0	1
229	M10	1	max	306.335	1	.68	4	1.427	5	.001	1	0	1	0	1
230			min	-343.951	3	.172	15	-.199	1	-.002	5	0	5	0	1
231		2	max	306.461	1	.629	4	1.312	5	.001	1	0	1	0	15
232			min	-343.856	3	.16	15	-.199	1	-.002	5	0	3	0	4
233		3	max	306.586	1	.578	4	1.198	5	.001	1	0	4	0	15
234			min	-343.762	3	.148	15	-.199	1	-.002	5	0	3	0	4
235		4	max	306.712	1	.527	4	1.083	5	.001	1	0	4	0	15
236			min	-343.667	3	.136	15	-.199	1	-.002	5	0	3	0	4
237		5	max	306.838	1	.476	4	.969	5	.001	1	0	4	0	15
238			min	-343.573	3	.124	15	-.199	1	-.002	5	0	3	0	4
239		6	max	306.964	1	.425	4	.854	5	.001	1	0	4	0	15
240			min	-343.479	3	.112	15	-.199	1	-.002	5	0	3	0	4
241		7	max	307.09	1	.373	4	.74	5	.001	1	.001	4	0	15
242			min	-343.384	3	.1	15	-.199	1	-.002	5	0	3	0	4
243		8	max	307.216	1	.322	4	.626	5	.001	1	.001	4	0	15
244			min	-343.29	3	.088	15	-.199	1	-.002	5	0	3	0	4
245		9	max	307.342	1	.271	4	.511	5	.001	1	.001	4	0	15
246			min	-343.195	3	.076	15	-.199	1	-.002	5	0	3	0	4
247		10	max	307.467	1	.22	4	.397	5	.001	1	.001	4	0	15
248			min	-343.101	3	.064	15	-.199	1	-.002	5	0	3	0	4
249		11	max	307.593	1	.169	4	.282	5	.001	1	.001	4	0	15
250			min	-343.007	3	.049	12	-.199	1	-.002	5	0	3	0	4
251		12	max	307.719	1	.118	4	.168	5	.001	1	.001	4	0	15
252			min	-342.912	3	.029	12	-.199	1	-.002	5	0	3	0	4
253		13	max	307.845	1	.067	4	.053	5	.001	1	.001	4	0	15
254			min	-342.818	3	.008	1	-.199	1	-.002	5	0	3	0	4
255		14	max	307.971	1	.023	5	-.003	12	.001	1	.001	4	0	15
256			min	-342.723	3	-.032	1	-.199	1	-.002	5	0	1	0	4



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	308.097	1	.004	5	-.003	12	.001	1	.001	4	0	15
258			min	-342.629	3	-.072	1	-.205	4	-.002	5	0	1	0	4
259		16	max	308.223	1	-.008	15	-.003	12	.001	1	.001	5	0	15
260			min	-342.535	3	-.112	1	-.32	4	-.002	5	0	1	0	4
261		17	max	308.349	1	-.02	15	-.003	12	.001	1	.001	5	0	15
262			min	-342.44	3	-.152	1	-.434	4	-.002	5	0	1	0	4
263		18	max	308.474	1	-.032	15	-.003	12	.001	1	.001	5	0	15
264			min	-342.346	3	-.192	1	-.549	4	-.002	5	0	1	0	4
265		19	max	308.6	1	-.044	15	-.003	12	.001	1	.001	5	0	15
266			min	-342.251	3	-.241	6	-.663	4	-.002	5	0	1	0	4
267	M11	1	max	156.077	2	1.746	6	.687	1	.002	4	.001	5	0	1
268			min	-177.157	3	.406	15	-1.19	5	0	10	-.002	1	0	15
269		2	max	156.008	2	1.569	6	.687	1	.002	4	0	5	0	1
270			min	-177.209	3	.364	15	-1.057	5	0	10	-.002	1	0	3
271		3	max	155.938	2	1.393	6	.687	1	.002	4	0	5	0	1
272			min	-177.261	3	.323	15	-.923	5	0	10	-.002	1	0	3
273		4	max	155.869	2	1.216	6	.687	1	.002	4	0	5	0	15
274			min	-177.313	3	.281	15	-.789	5	0	10	-.002	1	0	4
275		5	max	155.8	2	1.039	6	.687	1	.002	4	0	5	0	15
276			min	-177.365	3	.239	15	-.656	5	0	10	-.001	1	0	4
277		6	max	155.73	2	.862	6	.687	1	.002	4	0	3	0	15
278			min	-177.417	3	.198	15	-.522	5	0	10	-.001	1	0	4
279		7	max	155.661	2	.685	6	.687	1	.002	4	0	3	0	15
280			min	-177.469	3	.156	15	-.388	5	0	10	-.001	1	0	4
281		8	max	155.592	2	.508	6	.687	1	.002	4	0	3	0	15
282			min	-177.521	3	.115	15	-.255	5	0	10	0	1	-.001	4
283		9	max	155.522	2	.332	6	.687	1	.002	4	0	3	0	15
284			min	-177.573	3	.073	15	-.121	5	0	10	0	1	-.001	4
285		10	max	155.453	2	.155	1	.687	1	.002	4	0	3	0	15
286			min	-177.625	3	.032	15	0	3	0	10	0	1	-.001	4
287		11	max	155.384	2	.017	1	.687	1	.002	4	0	3	0	15
288			min	-177.677	3	-.057	3	0	3	0	10	0	1	-.001	4
289		12	max	155.314	2	-.052	15	.687	1	.002	4	0	3	0	15
290			min	-177.729	3	-.199	4	0	3	0	10	0	1	-.001	4
291		13	max	155.245	2	-.093	15	.687	1	.002	4	0	3	0	15
292			min	-177.781	3	-.376	4	0	3	0	10	0	1	-.001	4
293		14	max	155.176	2	-.135	15	.693	4	.002	4	0	3	0	15
294			min	-177.833	3	-.553	4	0	3	0	10	0	1	-.001	4
295		15	max	155.106	2	-.176	15	.827	4	.002	4	0	4	0	15
296			min	-177.885	3	-.73	4	0	3	0	10	0	10	0	4
297		16	max	155.037	2	-.218	15	.961	4	.002	4	0	4	0	15
298			min	-177.937	3	-.907	4	0	3	0	10	0	10	0	4
299		17	max	154.968	2	-.259	15	1.094	4	.002	4	0	4	0	15
300			min	-177.989	3	-1.084	4	0	3	0	10	0	10	0	4
301		18	max	154.898	2	-.301	15	1.228	4	.002	4	0	4	0	15
302			min	-178.041	3	-1.26	4	0	3	0	10	0	10	0	4
303		19	max	154.829	2	-.342	15	1.362	4	.002	4	.001	4	0	1
304			min	-178.093	3	-1.437	4	0	3	0	10	0	10	0	1
305	M12	1	max	436.051	1	0	1	3.913	1	0	1	0	4	0	1
306			min	5.805	12	0	1	-28.6	5	0	1	0	3	0	1
307		2	max	436.115	1	0	1	3.913	1	0	1	0	1	0	1
308			min	5.837	12	0	1	-28.656	5	0	1	-.003	5	0	1
309		3	max	436.18	1	0	1	3.913	1	0	1	0	1	0	1
310			min	5.87	12	0	1	-28.712	5	0	1	-.005	5	0	1
311		4	max	436.245	1	0	1	3.913	1	0	1	.001	1	0	1
312			min	5.902	12	0	1	-28.768	5	0	1	-.008	5	0	1
313		5	max	436.31	1	0	1	3.913	1	0	1	.001	1	0	1







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-9.01	10	-23.834	2	-28.303	5	0	5	-.077	1	-.142	3
429	6	max	90.056	3	6.433	9	73.799	1	0	1	.056	5	.128	1
430		min	-8.894	10	-24.076	2	-28.061	5	0	5	-.061	1	-.139	3
431	7	max	90.161	3	6.232	9	73.799	1	0	1	.05	5	.133	2
432		min	-8.777	10	-24.318	2	-27.819	5	0	5	-.045	1	-.136	3
433	8	max	90.265	3	6.03	9	73.799	1	0	1	.044	5	.138	2
434		min	-8.661	10	-24.56	2	-27.577	5	0	5	-.029	1	-.133	3
435	9	max	90.37	3	5.829	9	73.799	1	0	1	.038	5	.143	2
436		min	-8.545	10	-24.801	2	-27.335	5	0	5	-.013	1	-.13	3
437	10	max	90.475	3	5.627	9	73.799	1	0	1	.033	4	.149	2
438		min	-8.428	10	-25.043	2	-27.093	5	0	5	0	2	-.126	3
439	11	max	90.579	3	5.426	9	73.799	1	0	1	.03	4	.154	2
440		min	-8.312	10	-25.285	2	-26.851	5	0	5	.002	10	-.123	3
441	12	max	90.684	3	5.224	9	73.799	1	0	1	.035	1	.16	2
442		min	-8.196	10	-25.527	2	-26.609	5	0	5	.003	10	-.12	3
443	13	max	90.789	3	5.023	9	73.799	1	0	1	.051	1	.165	2
444		min	-8.079	10	-25.769	2	-26.367	5	0	5	.005	10	-.117	3
445	14	max	90.894	3	4.821	9	73.799	1	0	1	.067	1	.171	2
446		min	-7.963	10	-26.011	2	-26.125	5	0	5	.006	12	-.113	3
447	15	max	90.998	3	4.62	9	73.799	1	0	1	.083	1	.177	2
448		min	-7.847	10	-26.252	2	-25.883	5	0	5	.002	15	-.11	3
449	16	max	95.332	2	102.702	2	74.384	1	0	10	.1	1	.181	2
450		min	-5.947	3	-165.336	3	-24.431	5	0	4	0	15	-.105	3
451	17	max	95.472	2	102.46	2	74.384	1	0	10	.116	1	.159	2
452		min	-5.843	3	-165.517	3	-24.189	5	0	4	-.005	5	-.069	3
453	18	max	.184	15	368.351	2	78.383	1	0	2	.133	1	.08	2
454		min	-146.522	1	-159.093	3	-52.459	5	0	3	-.016	5	-.035	3
455	19	max	.226	15	368.109	2	78.383	1	0	2	.15	1	0	2
456		min	-146.382	1	-159.275	3	-52.217	5	0	3	-.028	5	0	3
457	M13	1	max	204.547	4	290.297	1	-3.983	15	0	.152	1	0	1
458		min	7.783	10	-344.794	3	-146.515	1	0	3	.002	15	0	3
459	2	max	196.635	4	204.762	1	-2.606	15	0	1	.048	1	.237	3
460		min	7.783	10	-243.128	3	-112.307	1	0	3	0	5	-.199	1
461	3	max	188.723	4	119.227	1	-1.229	15	0	1	.003	3	.392	3
462		min	7.783	10	-141.461	3	-78.099	1	0	3	-.028	1	-.33	1
463	4	max	180.811	4	33.692	1	.148	15	0	1	0	12	.465	3
464		min	7.783	10	-39.794	3	-43.891	1	0	3	-.078	1	-.391	1
465	5	max	172.899	4	61.872	3	2.249	5	0	1	-.002	15	.456	3
466		min	7.783	10	-51.842	1	-9.682	1	0	3	-.099	1	-.384	1
467	6	max	164.987	4	163.539	3	24.526	1	0	1	0	15	.365	3
468		min	7.783	10	-137.377	1	.194	12	0	3	-.093	1	-.308	1
469	7	max	157.075	4	265.205	3	58.734	1	0	1	.005	5	.192	3
470		min	7.783	10	-222.912	1	1.53	12	0	3	-.06	1	-.163	1
471	8	max	149.163	4	366.872	3	92.942	1	0	1	.011	4	.051	1
472		min	7.783	10	-308.446	1	2.866	12	0	3	0	3	-.062	3
473	9	max	141.252	4	468.539	3	127.15	1	0	1	.09	1	.334	1
474		min	7.783	10	-393.981	1	4.202	12	0	3	.003	12	-.399	3
475	10	max	133.34	4	570.205	3	161.359	1	0	2	.206	1	.686	1
476		min	7.783	10	-479.516	1	5.538	12	0	1	.007	12	-.817	3
477	11	max	99.172	4	393.981	1	-.37	15	0	3	.087	1	.334	1
478		min	3.939	12	-468.539	3	-126.485	1	0	1	-.014	5	-.399	3
479	12	max	91.26	4	308.446	1	1.279	5	0	3	.001	2	.051	1
480		min	3.939	12	-366.872	3	-92.276	1	0	1	-.015	4	-.062	3
481	13	max	83.348	4	222.912	1	3.409	5	0	3	-.004	12	.192	3
482		min	3.939	12	-265.205	3	-58.068	1	0	1	-.062	1	-.163	1
483	14	max	77.975	1	137.377	1	5.539	5	0	3	-.005	12	.365	3
484		min	3.939	12	-163.539	3	-23.86	1	0	1	-.095	1	-.308	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	77.975	1	51.842	1	10.348	1	0	3	-.002	15	.456	3
486		min	3.939	12	-61.872	3	.582	10	0	1	-.1	1	-.384	1
487	16	max	77.975	1	39.794	3	44.556	1	0	3	.004	5	.465	3
488		min	3.939	12	-33.693	1	2.653	12	0	1	-.078	1	-.391	1
489	17	max	77.975	1	141.461	3	78.765	1	0	3	.013	5	.392	3
490		min	3.939	12	-119.227	1	3.989	12	0	1	-.029	1	-.33	1
491	18	max	77.975	1	243.128	3	112.973	1	0	3	.049	1	.237	3
492		min	3.939	12	-204.762	1	5.325	12	0	1	.004	12	-.199	1
493	19	max	77.975	1	344.794	3	147.181	1	0	3	.153	1	0	1
494		min	3.939	12	-290.297	1	6.661	12	0	1	.008	12	0	3
495	M16	1	max	52.218	5	368.347	2	.226	15	0	.15	1	0	2
496		min	-78.083	1	-159.303	3	-146.398	1	0	2	-.028	5	0	3
497	2	max	44.306	5	259.828	2	2.113	5	0	3	.046	1	.109	3
498		min	-78.083	1	-112.49	3	-112.19	1	0	2	-.027	5	-.253	2
499	3	max	36.394	5	151.309	2	4.243	5	0	3	0	12	.181	3
500		min	-78.083	1	-65.677	3	-77.982	1	0	2	-.031	4	-.419	2
501	4	max	28.482	5	42.791	2	6.373	5	0	3	-.003	12	.215	3
502		min	-78.083	1	-18.865	3	-43.774	1	0	2	-.079	1	-.497	2
503	5	max	20.57	5	27.948	3	8.503	5	0	3	-.004	12	.212	3
504		min	-78.083	1	-65.728	2	-9.566	1	0	2	-.101	1	-.488	2
505	6	max	12.658	5	74.761	3	24.643	1	0	3	-.004	15	.17	3
506		min	-78.083	1	-174.247	2	.424	12	0	2	-.095	1	-.391	2
507	7	max	4.746	5	121.574	3	58.851	1	0	3	.003	5	.091	3
508		min	-78.083	1	-282.766	2	1.76	12	0	2	-.061	1	-.207	2
509	8	max	-1.398	12	168.387	3	93.059	1	0	3	.014	4	.065	2
510		min	-78.083	1	-391.285	2	3.096	12	0	2	-.003	3	-.026	3
511	9	max	-1.398	12	215.199	3	127.267	1	0	3	.089	1	.424	2
512		min	-78.083	1	-499.804	2	4.432	12	0	2	.001	12	-.18	3
513	10	max	28.959	5	-13.367	15	161.476	1	0	14	.205	1	.87	2
514		min	-79.82	1	-608.323	2	-8.976	3	0	2	.007	12	-.372	3
515	11	max	21.048	5	499.804	2	-.342	15	0	2	.089	1	.424	2
516		min	-79.82	1	-215.199	3	-126.918	1	0	3	-.013	5	-.18	3
517	12	max	13.136	5	391.285	2	1.318	5	0	2	.001	2	.065	2
518		min	-79.82	1	-168.387	3	-92.71	1	0	3	-.013	4	-.026	3
519	13	max	5.224	5	282.766	2	3.448	5	0	2	-.002	12	.091	3
520		min	-79.82	1	-121.574	3	-58.502	1	0	3	-.061	1	-.207	2
521	14	max	-1.677	15	174.247	2	5.578	5	0	2	-.004	12	.17	3
522		min	-79.82	1	-74.761	3	-24.294	1	0	3	-.094	1	-.391	2
523	15	max	-4.202	12	65.728	2	10.086	4	0	2	0	15	.212	3
524		min	-79.82	1	-27.948	3	.589	10	0	3	-.1	1	-.488	2
525	16	max	-4.202	12	18.865	3	44.123	1	0	2	.005	5	.215	3
526		min	-79.82	1	-42.791	2	2.014	12	0	3	-.078	1	-.497	2
527	17	max	-4.202	12	65.677	3	78.331	1	0	2	.014	5	.181	3
528		min	-79.82	1	-151.31	2	3.349	12	0	3	-.029	1	-.419	2
529	18	max	-4.202	12	112.49	3	112.539	1	0	2	.048	1	.109	3
530		min	-79.82	1	-259.828	2	4.685	12	0	3	.003	12	-.253	2
531	19	max	-4.202	12	159.303	3	146.748	1	0	2	.153	1	0	2
532		min	-79.82	1	-368.347	2	6.021	12	0	3	.007	12	0	3
533	M15	1	max	0	1.976	1	.04	3	0	1	0	1	0	1
534		min	-51.731	3	0	2	-.037	1	0	3	0	3	0	1
535	2	max	0	2	1.756	1	.04	3	0	1	0	1	0	2
536		min	-51.801	3	0	2	-.037	1	0	3	0	3	0	1
537	3	max	0	2	1.537	1	.04	3	0	1	0	1	0	2
538		min	-51.872	3	0	2	-.037	1	0	3	0	3	-.002	1
539	4	max	0	2	1.317	1	.04	3	0	1	0	1	0	2
540		min	-51.942	3	0	2	-.037	1	0	3	0	3	-.002	1
541	5	max	0	2	1.098	1	.04	3	0	1	0	1	0	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-52.013	3	0	2	-.037	1	0	3	0	3	-.003	1
543		6	max	0	2	.878	1	.04	3	0	1	0	1	0	2
544			min	-52.083	3	0	2	-.037	1	0	3	0	3	-.003	1
545		7	max	0	2	.659	1	.04	3	0	1	0	3	0	2
546			min	-52.154	3	0	2	-.037	1	0	3	0	1	-.003	1
547		8	max	0	2	.439	1	.04	3	0	1	0	3	0	2
548			min	-52.224	3	0	2	-.037	1	0	3	0	1	-.004	1
549		9	max	0	2	.22	1	.04	3	0	1	0	3	0	2
550			min	-52.295	3	0	2	-.037	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.04	3	0	1	0	3	0	2
552			min	-52.365	3	0	1	-.037	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.04	3	0	1	0	3	0	2
554			min	-52.436	3	-.22	1	-.037	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.04	3	0	1	0	3	0	2
556			min	-52.506	3	-.439	1	-.037	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.04	3	0	1	0	3	0	2
558			min	-52.577	3	-.659	1	-.037	1	0	3	0	1	-.003	1
559		14	max	0	2	0	2	.04	3	0	1	0	3	0	2
560			min	-52.647	3	-.878	1	-.037	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.04	3	0	1	0	3	0	2
562			min	-52.718	3	-1.098	1	-.037	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.04	3	0	1	0	3	0	2
564			min	-52.788	3	-1.317	1	-.037	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.04	3	0	1	0	3	0	2
566			min	-52.859	3	-1.537	1	-.037	1	0	3	0	1	-.002	1
567		18	max	0	2	0	2	.04	3	0	1	0	3	0	2
568			min	-52.929	3	-1.756	1	-.037	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.04	3	0	1	0	3	0	1
570			min	-53	3	-1.976	1	-.037	1	0	3	0	1	0	1
571	M16A	1	max	-.887	10	3.262	4	.283	4	0	3	0	3	0	1
572			min	-241.707	4	.988	12	-.016	3	0	2	0	4	0	1
573		2	max	-.808	10	2.899	4	.255	4	0	3	0	3	0	12
574			min	-241.775	4	.878	12	-.016	3	0	2	0	4	-.001	4
575		3	max	-.73	10	2.537	4	.227	4	0	3	0	3	0	12
576			min	-241.843	4	.768	12	-.016	3	0	2	0	4	-.003	4
577		4	max	-.652	10	2.175	4	.199	4	0	3	0	3	-.001	12
578			min	-241.911	4	.659	12	-.016	3	0	2	0	4	-.004	4
579		5	max	-.573	10	1.812	4	.171	4	0	3	0	3	-.001	12
580			min	-241.979	4	.549	12	-.016	3	0	2	0	1	-.004	4
581		6	max	-.495	10	1.45	4	.143	4	0	3	0	5	-.002	12
582			min	-242.047	4	.439	12	-.016	3	0	2	0	1	-.005	4
583		7	max	-.417	10	1.087	4	.115	4	0	3	0	5	-.002	12
584			min	-242.115	4	.329	12	-.016	3	0	2	0	1	-.006	4
585		8	max	-.338	10	.725	4	.087	4	0	3	0	5	-.002	12
586			min	-242.183	4	.22	12	-.016	3	0	2	0	1	-.006	4
587		9	max	-.26	10	.362	4	.058	4	0	3	0	5	-.002	12
588			min	-242.25	4	.11	12	-.016	3	0	2	0	1	-.006	4
589		10	max	-.182	10	0	1	.03	4	0	3	0	5	-.002	12
590			min	-242.318	4	0	1	-.016	3	0	2	0	1	-.006	4
591		11	max	-.104	10	-.11	12	.023	1	0	3	0	5	-.002	12
592			min	-242.386	4	-.362	4	-.016	3	0	2	0	1	-.006	4
593		12	max	-.025	10	-.22	12	.023	1	0	3	0	5	-.002	12
594			min	-242.454	4	-.725	4	-.03	5	0	2	0	1	-.006	4
595		13	max	.053	10	-.329	12	.023	1	0	3	0	5	-.002	12
596			min	-242.522	4	-1.087	4	-.058	5	0	2	0	3	-.006	4
597		14	max	.131	10	-.439	12	.023	1	0	3	0	4	-.002	12
598			min	-242.59	4	-1.45	4	-.086	5	0	2	0	3	-.005	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.21	10	-.549	12	.023	1	0	3	0	4	-.001	12
600		min	-242.658	4	-1.812	4	-.114	5	0	2	0	3	-.004	4
601	16	max	.288	10	-.659	12	.023	1	0	3	0	4	-.001	12
602		min	-242.726	4	-2.175	4	-.142	5	0	2	0	3	-.004	4
603	17	max	.366	10	-.768	12	.023	1	0	3	0	1	0	12
604		min	-242.793	4	-2.537	4	-.17	5	0	2	0	3	-.003	4
605	18	max	.445	10	-.878	12	.023	1	0	3	0	1	0	12
606		min	-242.861	4	-2.899	4	-.198	5	0	2	0	5	-.001	4
607	19	max	.523	10	-.988	12	.023	1	0	3	0	1	0	1
608		min	-242.929	4	-3.262	4	-.226	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.009	2	.015	1	1.845e-3	5	NC	3	NC	3	
2			min	-.004	3	-.009	3	-.017	5	-1.252e-3	1	4195.741	2	2650.555	1	
3			2	max	.003	1	.009	2	.014	1	1.868e-3	5	NC	3	NC	3
4				min	-.003	3	-.009	3	-.017	5	-1.198e-3	1	4577.808	2	2853.769	1
5			3	max	.003	1	.008	2	.013	1	1.891e-3	5	NC	3	NC	3
6				min	-.003	3	-.009	3	-.016	5	-1.144e-3	1	5031.931	2	3093.889	1
7			4	max	.002	1	.007	2	.012	1	1.913e-3	5	NC	1	NC	3
8				min	-.003	3	-.008	3	-.016	5	-1.09e-3	1	5575.41	2	3379.709	1
9			5	max	.002	1	.006	2	.011	1	1.936e-3	5	NC	1	NC	3
10				min	-.003	3	-.008	3	-.015	5	-1.036e-3	1	6231.274	2	3722.964	1
11			6	max	.002	1	.006	2	.01	1	1.959e-3	5	NC	1	NC	2
12				min	-.003	3	-.007	3	-.015	5	-9.816e-4	1	7030.68	2	4139.601	1
13			7	max	.002	1	.005	2	.008	1	1.982e-3	5	NC	1	NC	2
14				min	-.002	3	-.007	3	-.014	5	-9.274e-4	1	8016.577	2	4651.733	1
15			8	max	.002	1	.004	2	.007	1	2.005e-3	5	NC	1	NC	2
16				min	-.002	3	-.007	3	-.013	5	-8.732e-4	1	9249.487	2	5290.76	1
17			9	max	.002	1	.004	2	.006	1	2.027e-3	5	NC	1	NC	2
18				min	-.002	3	-.006	3	-.012	5	-8.191e-4	1	NC	1	6102.516	1
19			10	max	.001	1	.003	2	.006	1	2.05e-3	5	NC	1	NC	2
20				min	-.002	3	-.006	3	-.011	5	-7.649e-4	1	NC	1	7156.144	1
21		11	max	.001	1	.003	2	.005	1	2.073e-3	5	NC	1	NC	2	
22			min	-.002	3	-.005	3	-.01	5	-7.108e-4	1	NC	1	8560.127	1	
23		12	max	.001	1	.002	2	.004	1	2.096e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.009	5	-6.566e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.003	1	2.119e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.008	5	-6.025e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.141e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.007	5	-5.483e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.164e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.006	5	-4.941e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	2.187e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.004	5	-4.4e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.21e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.003	5	-3.858e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.232e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.002	5	-3.317e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.255e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.775e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.31e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.063e-3	5	NC	1	NC	1	
41			2	max	0	3	0	2	.005	5	1.608e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-1.078e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.011	5	1.907e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.092e-3	5	NC	1	9049.148	14
45		4	max	0	3	0	2	.016	5	2.206e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	-.001	1	-1.106e-3	5	NC	1	5918.183	14
47		5	max	0	3	0	2	.022	5	2.505e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	-1.12e-3	5	NC	1	4365.536	14
49		6	max	0	3	0	2	.027	5	2.804e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.134e-3	5	NC	1	3442.863	14
51		7	max	0	3	0	2	.033	4	3.103e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.149e-3	5	NC	1	2834.233	14
53		8	max	0	3	0	2	.038	4	3.402e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.163e-3	5	NC	1	2404.367	14
55		9	max	0	3	.001	2	.044	4	3.701e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-1.177e-3	5	NC	1	2085.709	14
57		10	max	.001	3	.002	2	.049	4	4.e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-1.191e-3	5	NC	1	1840.787	14
59		11	max	.001	3	.002	2	.054	4	4.299e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-1.205e-3	5	NC	1	1647.159	14
61		12	max	.001	3	.003	2	.059	4	4.598e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	12	-1.219e-3	5	NC	1	1490.57	14
63		13	max	.001	3	.004	2	.065	4	4.897e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	12	-1.234e-3	5	NC	1	1361.541	14
65		14	max	.001	3	.004	2	.07	4	5.195e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	12	-1.248e-3	5	NC	1	1253.52	14
67		15	max	.002	3	.005	2	.075	4	5.494e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	12	-1.262e-3	5	8813.519	2	1161.84	14
69		16	max	.002	3	.006	2	.08	4	5.793e-4	1	NC	1	NC	2
70			min	-.001	2	-.008	3	0	12	-1.276e-3	5	7475.071	2	1083.084	14
71		17	max	.002	3	.007	2	.085	4	6.092e-4	1	NC	1	NC	2
72			min	-.002	2	-.008	3	0	12	-1.29e-3	5	6437.905	2	1014.692	14
73		18	max	.002	3	.008	2	.09	4	6.391e-4	1	NC	3	NC	2
74			min	-.002	2	-.008	3	0	12	-1.305e-3	5	5625.272	2	954.712	14
75		19	max	.002	3	.009	2	.094	4	6.69e-4	1	NC	3	NC	2
76			min	-.002	2	-.008	3	0	12	-1.319e-3	5	4983.018	2	901.623	14
77	M4	1	max	.002	1	.011	2	0	12	6.633e-3	5	NC	1	NC	3
78			min	0	12	-.009	3	-.1	4	-9.714e-4	1	NC	1	193.605	4
79		2	max	.002	1	.01	2	0	12	6.633e-3	5	NC	1	NC	3
80			min	0	12	-.009	3	-.092	4	-9.714e-4	1	NC	1	211.057	4
81		3	max	.002	1	.01	2	0	12	6.633e-3	5	NC	1	NC	2
82			min	0	12	-.008	3	-.083	4	-9.714e-4	1	NC	1	231.829	4
83		4	max	.002	1	.009	2	0	12	6.633e-3	5	NC	1	NC	2
84			min	0	12	-.008	3	-.075	4	-9.714e-4	1	NC	1	256.796	4
85		5	max	.002	1	.008	2	0	12	6.633e-3	5	NC	1	NC	2
86			min	0	12	-.007	3	-.067	4	-9.714e-4	1	NC	1	287.149	4
87		6	max	.002	1	.008	2	0	12	6.633e-3	5	NC	1	NC	2
88			min	0	12	-.007	3	-.06	4	-9.714e-4	1	NC	1	324.546	4
89		7	max	.001	1	.007	2	0	12	6.633e-3	5	NC	1	NC	2
90			min	0	12	-.006	3	-.052	4	-9.714e-4	1	NC	1	371.346	4
91		8	max	.001	1	.007	2	0	12	6.633e-3	5	NC	1	NC	2
92			min	0	12	-.006	3	-.045	4	-9.714e-4	1	NC	1	431.004	4
93		9	max	.001	1	.006	2	0	12	6.633e-3	5	NC	1	NC	2
94			min	0	12	-.005	3	-.038	4	-9.714e-4	1	NC	1	508.748	4
95		10	max	.001	1	.005	2	0	12	6.633e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.032	4	-9.714e-4	1	NC	1	612.792	4
97		11	max	0	1	.005	2	0	12	6.633e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.026	4	-9.714e-4	1	NC	1	756.669	4
99		12	max	0	1	.004	2	0	12	6.633e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.004	3	-.02	4	-9.714e-4	1	NC	1	963.981	4
101		max	0	1	.004	2	0	12	6.633e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.015	4	-9.714e-4	1	NC	1	1279.002	4
103		max	0	1	.003	2	0	12	6.633e-3	5	NC	1	NC	1
104		min	0	12	-.003	3	-.011	4	-9.714e-4	1	NC	1	1793.058	4
105		max	0	1	.002	2	0	12	6.633e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.007	4	-9.714e-4	1	NC	1	2720.702	4
107		max	0	1	.002	2	0	12	6.633e-3	5	NC	1	NC	1
108		min	0	12	-.002	3	-.004	4	-9.714e-4	1	NC	1	4671.873	4
109		max	0	1	.001	2	0	12	6.633e-3	5	NC	1	NC	1
110		min	0	12	-.001	3	-.002	4	-9.714e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	12	6.633e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-9.714e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	6.633e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-9.714e-4	1	NC	1	NC	1
115	M6	max	.009	1	.034	2	.005	1	2.027e-3	4	NC	3	NC	2
116		min	-.012	3	-.03	3	-.017	5	1.216e-6	10	1153.295	2	8120.666	1
117		max	.009	1	.032	2	.004	1	2.045e-3	4	NC	3	NC	2
118		min	-.011	3	-.029	3	-.017	5	5.191e-7	10	1233.189	2	8803.252	1
119		max	.008	1	.03	2	.004	1	2.064e-3	4	NC	3	NC	2
120		min	-.01	3	-.027	3	-.017	5	-1.773e-7	10	1324.62	2	9610.829	1
121		max	.008	1	.028	2	.004	1	2.082e-3	4	NC	3	NC	1
122		min	-.01	3	-.025	3	-.016	5	-8.738e-7	10	1429.891	2	NC	1
123		max	.007	1	.025	2	.003	1	2.101e-3	4	NC	3	NC	1
124		min	-.009	3	-.024	3	-.016	5	-3.587e-6	2	1551.968	2	NC	1
125		max	.007	1	.023	2	.003	1	2.119e-3	4	NC	3	NC	1
126		min	-.008	3	-.022	3	-.015	5	-6.613e-6	2	1694.731	2	NC	1
127		max	.006	1	.021	2	.003	1	2.138e-3	4	NC	3	NC	1
128		min	-.008	3	-.021	3	-.014	5	-9.639e-6	2	1863.357	2	NC	1
129		max	.006	1	.019	2	.002	1	2.156e-3	4	NC	3	NC	1
130		min	-.007	3	-.019	3	-.014	5	-1.267e-5	2	2064.909	2	NC	1
131		max	.005	1	.017	2	.002	1	2.175e-3	4	NC	3	NC	1
132		min	-.006	3	-.017	3	-.013	5	-1.569e-5	2	2309.271	2	NC	1
133		max	.005	1	.015	2	.002	1	2.193e-3	4	NC	3	NC	1
134		min	-.006	3	-.016	3	-.012	5	-1.872e-5	2	2610.722	2	NC	1
135		max	.004	1	.013	2	.001	1	2.212e-3	4	NC	3	NC	1
136		min	-.005	3	-.014	3	-.011	5	-2.174e-5	2	2990.67	2	NC	1
137		max	.004	1	.011	2	.001	1	2.23e-3	4	NC	3	NC	1
138		min	-.005	3	-.012	3	-.01	5	-2.477e-5	2	3482.749	2	NC	1
139		max	.003	1	.01	2	0	1	2.249e-3	4	NC	3	NC	1
140		min	-.004	3	-.011	3	-.009	5	-2.78e-5	2	4143.001	2	NC	1
141		max	.003	1	.008	2	0	1	2.267e-3	4	NC	3	NC	1
142		min	-.003	3	-.009	3	-.007	5	-3.082e-5	2	5072.272	2	NC	1
143		max	.002	1	.006	2	0	1	2.286e-3	4	NC	3	NC	1
144		min	-.003	3	-.007	3	-.006	5	-3.385e-5	2	6472.216	2	NC	1
145		max	.002	1	.004	2	0	1	2.304e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.005	5	-3.687e-5	2	8813.284	2	NC	1
147		max	.001	1	.003	2	0	1	2.323e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.003	5	-3.99e-5	2	NC	1	NC	1
149		max	0	1	.001	2	0	1	2.341e-3	4	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-4.293e-5	2	NC	1	NC	1
151		max	0	1	0	1	0	1	2.361e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.214e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.416e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.113e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.006	5	2.242e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-1.113e-3	4	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.011	5	2.069e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.113e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.017	5	1.895e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.114e-3	4	9938.134	2	NC	1
161		5	max	.001	3	.006	2	.023	5	1.722e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-1.114e-3	4	7507.779	2	NC	1
163		6	max	.002	3	.008	2	.028	4	2.102e-5	3	NC	3	NC	1
164			min	-.002	2	-.01	3	0	1	-1.115e-3	4	6020.214	2	NC	1
165		7	max	.002	3	.009	2	.034	4	3.777e-5	3	NC	3	NC	1
166			min	-.002	2	-.012	3	0	1	-1.115e-3	4	5006.135	2	NC	1
167		8	max	.002	3	.011	2	.039	4	5.451e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-1.115e-3	4	4264.926	2	NC	1
169		9	max	.003	3	.012	2	.045	4	7.126e-5	3	NC	3	NC	1
170			min	-.003	2	-.015	3	-.001	1	-1.116e-3	4	3696.566	2	NC	1
171		10	max	.003	3	.014	2	.05	4	8.8e-5	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-1.116e-3	4	3245.602	2	NC	1
173		11	max	.003	3	.016	2	.056	4	1.047e-4	3	NC	3	NC	1
174			min	-.004	2	-.018	3	-.001	1	-1.116e-3	4	2878.759	2	NC	1
175		12	max	.004	3	.018	2	.061	4	1.215e-4	3	NC	3	NC	1
176			min	-.004	2	-.019	3	-.001	1	-1.117e-3	4	2574.786	2	NC	1
177		13	max	.004	3	.02	2	.066	4	1.382e-4	3	NC	3	NC	1
178			min	-.005	2	-.021	3	-.002	1	-1.117e-3	4	2319.395	2	NC	1
179		14	max	.005	3	.022	2	.071	4	1.55e-4	3	NC	3	NC	1
180			min	-.005	2	-.022	3	-.002	1	-1.117e-3	4	2102.563	2	NC	1
181		15	max	.005	3	.024	2	.076	4	1.717e-4	3	NC	3	NC	1
182			min	-.006	2	-.023	3	-.002	1	-1.118e-3	4	1916.996	2	NC	1
183		16	max	.005	3	.026	2	.08	4	1.885e-4	3	NC	3	NC	1
184			min	-.006	2	-.024	3	-.002	1	-1.118e-3	4	1757.227	2	NC	1
185		17	max	.006	3	.028	2	.085	4	2.052e-4	3	NC	3	NC	1
186			min	-.006	2	-.025	3	-.002	1	-1.119e-3	4	1619.052	2	NC	1
187		18	max	.006	3	.031	2	.09	4	2.22e-4	3	NC	3	NC	1
188			min	-.007	2	-.026	3	-.002	1	-1.119e-3	4	1499.174	2	NC	1
189		19	max	.006	3	.033	2	.095	4	2.387e-4	3	NC	3	NC	1
190			min	-.007	2	-.027	3	-.002	1	-1.119e-3	4	1394.966	2	NC	1
191	M8	1	max	.005	1	.039	2	.002	1	6.451e-3	4	NC	1	NC	2
192			min	0	3	-.03	3	-.1	4	-1.893e-4	3	NC	1	193.532	4
193		2	max	.005	1	.037	2	.002	1	6.451e-3	4	NC	1	NC	2
194			min	0	3	-.028	3	-.092	4	-1.893e-4	3	NC	1	210.975	4
195		3	max	.005	1	.035	2	.002	1	6.451e-3	4	NC	1	NC	2
196			min	0	3	-.027	3	-.083	4	-1.893e-4	3	NC	1	231.738	4
197		4	max	.005	1	.033	2	.002	1	6.451e-3	4	NC	1	NC	1
198			min	0	3	-.025	3	-.075	4	-1.893e-4	3	NC	1	256.694	4
199		5	max	.004	1	.03	2	.002	1	6.451e-3	4	NC	1	NC	1
200			min	0	3	-.023	3	-.067	4	-1.893e-4	3	NC	1	287.035	4
201		6	max	.004	1	.028	2	.001	1	6.451e-3	4	NC	1	NC	1
202			min	0	3	-.022	3	-.06	4	-1.893e-4	3	NC	1	324.415	4
203		7	max	.004	1	.026	2	.001	1	6.451e-3	4	NC	1	NC	1
204			min	0	3	-.02	3	-.052	4	-1.893e-4	3	NC	1	371.194	4
205		8	max	.003	1	.024	2	.001	1	6.451e-3	4	NC	1	NC	1
206			min	0	3	-.018	3	-.045	4	-1.893e-4	3	NC	1	430.827	4
207		9	max	.003	1	.022	2	0	1	6.451e-3	4	NC	1	NC	1
208			min	0	3	-.017	3	-.038	4	-1.893e-4	3	NC	1	508.536	4
209		10	max	.003	1	.02	2	0	1	6.451e-3	4	NC	1	NC	1
210			min	0	3	-.015	3	-.032	4	-1.893e-4	3	NC	1	612.535	4
211		11	max	.002	1	.017	2	0	1	6.451e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.026	4	-1.893e-4	3	NC	1	756.349	4
213		12	max	.002	1	.015	2	0	1	6.451e-3	4	NC	1	NC	1



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 30° 90mph 30psf 7.25ft 7-05.rdb Page 36





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	12	-.004	3	-.018	5	8.825e-5	10	NC	1	1052.517	5
329		max	0	1	.004	2	.002	1	7.908e-3	4	NC	1	NC	1
330		min	0	12	-.003	3	-.014	5	8.825e-5	10	NC	1	1396.429	5
331		max	0	1	.003	2	.001	1	7.908e-3	4	NC	1	NC	1
332		min	0	12	-.003	3	-.01	5	8.825e-5	10	NC	1	1957.619	5
333		max	0	1	.002	2	0	1	7.908e-3	4	NC	1	NC	1
334		min	0	12	-.002	3	-.007	5	8.825e-5	10	NC	1	2970.304	5
335		max	0	1	.002	2	0	1	7.908e-3	4	NC	1	NC	1
336		min	0	12	-.002	3	-.004	5	8.825e-5	10	NC	1	5100.311	5
337		max	0	1	.001	2	0	1	7.908e-3	4	NC	1	NC	1
338		min	0	12	-.001	3	-.002	5	8.825e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	7.908e-3	4	NC	1	NC	1
340		min	0	12	0	3	0	5	8.825e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	7.908e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	8.825e-5	10	NC	1	NC	1
343	M1	max	.008	3	.025	3	.009	5	2.09e-2	1	NC	1	NC	1
344		min	-.009	2	-.023	2	-.005	1	-2.468e-2	3	NC	1	NC	1
345		max	.008	3	.015	3	.013	5	9.979e-3	1	NC	4	NC	2
346		min	-.009	2	-.013	2	-.011	1	-1.222e-2	3	4622.042	2	7820.557	1
347		max	.008	3	.005	3	.017	5	6.206e-4	5	NC	4	NC	2
348		min	-.009	2	-.004	2	-.015	1	-7.353e-4	1	2372.433	2	4744.195	1
349		max	.008	3	.004	1	.022	5	6.35e-4	5	NC	4	NC	2
350		min	-.009	2	-.003	3	-.017	1	-6.296e-4	1	1659.757	2	3597.242	5
351		max	.008	3	.011	2	.027	5	6.495e-4	5	NC	5	NC	3
352		min	-.009	2	-.009	3	-.017	1	-5.238e-4	1	1314.932	2	2579.909	5
353		max	.008	3	.017	2	.033	5	6.639e-4	5	NC	5	NC	2
354		min	-.009	2	-.015	3	-.016	1	-4.181e-4	1	1117.991	2	1985.349	5
355		max	.008	3	.022	2	.038	5	6.783e-4	5	NC	5	NC	2
356		min	-.009	2	-.019	3	-.014	1	-3.123e-4	1	996.603	2	1599.847	5
357		max	.008	3	.025	2	.044	5	6.928e-4	5	NC	5	NC	2
358		min	-.009	2	-.022	3	-.012	1	-2.066e-4	1	920.55	2	1332.307	5
359		max	.008	3	.027	2	.05	5	7.072e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.008	1	-1.009e-4	1	875.566	2	1134.468	4
361		max	.008	3	.028	2	.056	5	7.217e-4	5	NC	5	NC	1
362		min	-.009	2	-.024	3	-.005	1	6.766e-8	11	854.852	2	970.163	4
363		max	.008	3	.028	2	.063	4	7.601e-4	4	NC	5	NC	1
364		min	-.009	2	-.023	3	-.001	1	1.813e-5	10	855.965	2	846.827	4
365		max	.008	3	.026	2	.07	4	7.99e-4	4	NC	5	NC	2
366		min	-.009	2	-.021	3	0	10	2.821e-5	10	879.905	2	752.141	4
367		max	.008	3	.023	2	.076	4	8.379e-4	4	NC	5	NC	2
368		min	-.009	2	-.018	3	0	12	3.318e-5	12	931.658	2	678.22	4
369		max	.008	3	.018	2	.083	4	8.768e-4	4	NC	5	NC	2
370		min	-.009	2	-.014	3	0	12	3.671e-5	12	1022.717	2	619.833	4
371		max	.008	3	.011	2	.089	4	9.156e-4	4	NC	4	NC	2
372		min	-.009	2	-.009	3	0	12	4.023e-5	12	1178.295	2	573.407	4
373		max	.008	3	.003	2	.095	4	1.293e-3	4	NC	4	NC	2
374		min	-.009	2	-.003	3	0	12	4.267e-5	12	1460.017	2	536.451	4
375		max	.008	3	.004	3	.1	4	9.726e-3	4	NC	4	NC	2
376		min	-.009	2	-.007	2	0	12	-7.95e-5	1	2064.711	2	507.226	4
377		max	.008	3	.012	3	.104	4	1.313e-2	2	NC	4	NC	2
378		min	-.009	2	-.018	2	0	10	-5.781e-3	3	3999.039	2	484.434	4
379		max	.008	3	.02	3	.108	4	2.654e-2	2	NC	1	NC	1
380		min	-.009	2	-.03	2	-.003	1	-1.17e-2	3	NC	1	467.728	4
381	M5	max	.027	3	.082	3	.009	5	8.315e-6	4	NC	1	NC	1
382		min	-.032	2	-.078	2	-.006	1	4.908e-8	10	NC	1	NC	1
383		max	.027	3	.048	3	.013	5	3.141e-4	5	NC	5	NC	1
384		min	-.032	2	-.045	2	-.005	1	-6.527e-5	1	1374.048	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.027	3	.017	3	.017	5	6.149e-4	5	NC	5	NC	1
386			min	-.032	2	-.014	2	-.005	1	-1.3e-4	1	704.844	2	NC	1
387		4	max	.027	3	.013	2	.023	5	6.4e-4	5	NC	5	NC	1
388			min	-.032	2	-.009	3	-.004	1	-1.235e-4	1	492.569	2	NC	1
389		5	max	.027	3	.037	2	.028	5	6.651e-4	5	NC	5	NC	1
390			min	-.032	2	-.03	3	-.004	1	-1.171e-4	1	389.809	2	NC	1
391		6	max	.027	3	.057	2	.034	5	6.902e-4	5	NC	15	NC	1
392			min	-.032	2	-.048	3	-.004	1	-1.106e-4	1	331.083	2	NC	1
393		7	max	.027	3	.073	2	.04	5	7.154e-4	5	NC	15	NC	1
394			min	-.032	2	-.061	3	-.003	1	-1.042e-4	1	294.849	2	NC	1
395		8	max	.027	3	.085	2	.047	5	7.405e-4	5	NC	15	NC	1
396			min	-.032	2	-.07	3	-.003	1	-9.774e-5	1	272.105	2	NC	1
397		9	max	.026	3	.092	2	.053	5	7.656e-4	5	NC	15	NC	1
398			min	-.032	2	-.075	3	-.003	1	-9.129e-5	1	258.597	2	NC	1
399		10	max	.026	3	.096	2	.059	5	7.907e-4	5	NC	15	NC	1
400			min	-.032	2	-.077	3	-.003	1	-8.483e-5	1	252.294	2	NC	1
401		11	max	.026	3	.094	2	.066	5	8.158e-4	5	NC	15	NC	1
402			min	-.032	2	-.074	3	-.002	1	-7.838e-5	1	252.462	2	NC	1
403		12	max	.026	3	.088	2	.072	5	8.409e-4	5	NC	15	NC	1
404			min	-.032	2	-.068	3	-.002	1	-7.193e-5	1	259.384	2	NC	1
405		13	max	.026	3	.077	2	.078	5	8.661e-4	5	NC	15	NC	1
406			min	-.032	2	-.058	3	-.002	1	-6.547e-5	1	274.528	2	NC	1
407		14	max	.026	3	.06	2	.084	4	8.912e-4	5	NC	15	NC	1
408			min	-.032	2	-.045	3	-.002	1	-5.902e-5	1	301.288	2	NC	1
409		15	max	.026	3	.038	2	.09	4	9.163e-4	5	NC	5	NC	1
410			min	-.032	2	-.028	3	-.002	1	-5.257e-5	1	347.125	2	NC	1
411		16	max	.026	3	.011	2	.095	4	1.278e-3	5	NC	5	NC	1
412			min	-.032	2	-.009	3	-.002	1	-5.371e-5	1	430.334	2	NC	1
413		17	max	.026	3	.015	3	.1	4	9.707e-3	4	NC	5	NC	1
414			min	-.032	2	-.023	2	-.002	1	-2.356e-4	1	609.863	2	NC	1
415		18	max	.026	3	.04	3	.104	4	4.978e-3	4	NC	5	NC	1
416			min	-.032	2	-.061	2	-.003	1	-1.206e-4	1	1182.369	2	NC	1
417		19	max	.026	3	.066	3	.108	4	2.247e-6	5	NC	1	NC	1
418			min	-.032	2	-.102	2	-.003	1	-2.931e-7	3	NC	1	NC	1
419	M9	1	max	.008	3	.025	3	.007	5	2.468e-2	3	NC	1	NC	1
420			min	-.009	2	-.023	2	-.007	1	-2.09e-2	1	NC	1	NC	1
421		2	max	.008	3	.015	3	.007	5	1.221e-2	3	NC	4	NC	2
422			min	-.009	2	-.013	2	-.001	1	-1.023e-2	1	4624.12	2	8887.218	1
423		3	max	.008	3	.005	3	.007	4	2.455e-4	1	NC	4	NC	2
424			min	-.009	2	-.004	2	0	3	-3.22e-5	3	2373.527	2	5500.071	1
425		4	max	.008	3	.004	2	.01	4	1.561e-4	1	NC	4	NC	2
426			min	-.009	2	-.003	3	0	3	-3.934e-5	3	1660.537	2	4645.794	1
427		5	max	.008	3	.011	2	.012	4	6.66e-5	1	NC	4	NC	2
428			min	-.009	2	-.009	3	-.002	3	-4.648e-5	3	1315.548	2	4584.997	1
429		6	max	.008	3	.017	2	.016	4	5.306e-5	4	NC	5	NC	2
430			min	-.009	2	-.015	3	-.002	3	-5.363e-5	3	1118.507	2	4320.441	4
431		7	max	.008	3	.022	2	.02	4	6.931e-5	5	NC	5	NC	2
432			min	-.009	2	-.019	3	-.003	3	-1.123e-4	1	997.056	2	3120.432	4
433		8	max	.008	3	.025	2	.025	4	8.638e-5	5	NC	5	NC	1
434			min	-.009	2	-.022	3	-.003	3	-2.017e-4	1	920.961	2	2366.456	4
435		9	max	.008	3	.027	2	.03	5	1.035e-4	5	NC	5	NC	1
436			min	-.009	2	-.023	3	-.003	3	-2.912e-4	1	875.948	2	1862.334	4
437		10	max	.008	3	.028	2	.037	5	1.205e-4	5	NC	5	NC	1
438			min	-.009	2	-.024	3	-.006	1	-3.806e-4	1	855.217	2	1508.663	4
439		11	max	.008	3	.028	2	.044	5	1.376e-4	5	NC	5	NC	2
440			min	-.009	2	-.023	3	-.009	1	-4.701e-4	1	856.322	2	1250.89	4
441		12	max	.008	3	.026	2	.051	5	1.547e-4	5	NC	5	NC	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.22	3	.113	1	7.015e-3	2	NC	5	NC	3
500		min	-.108	4	-.48	2	.006	10	-4.629e-3	3	386.31	2	1473.823	1
501	4	max	.002	1	.277	3	.171	1	8.232e-3	2	NC	15	NC	10
502		min	-.108	4	-.607	2	.011	10	-5.398e-3	3	301.329	2	990.36	1
503	5	max	.002	1	.295	3	.199	1	9.448e-3	2	NC	15	NC	10
504		min	-.108	4	-.644	2	.012	10	-6.167e-3	3	283.158	2	853.998	1
505	6	max	.002	1	.274	3	.189	1	1.066e-2	2	NC	15	NC	10
506		min	-.108	4	-.593	2	.009	10	-6.936e-3	3	308.903	2	897.052	1
507	7	max	.002	1	.223	3	.144	1	1.188e-2	2	NC	5	NC	5
508		min	-.108	4	-.472	2	.003	10	-7.705e-3	3	394.057	2	1170.219	1
509	8	max	.003	1	.156	3	.076	1	1.31e-2	2	NC	5	NC	5
510		min	-.108	4	-.313	2	-.006	10	-8.474e-3	3	614.474	2	2152.641	1
511	9	max	.003	1	.094	3	.029	3	1.431e-2	2	NC	5	NC	1
512		min	-.108	4	-.168	2	-.019	2	-9.243e-3	3	1260.775	2	8418.885	3
513	10	max	.003	1	.066	3	.026	3	1.553e-2	2	NC	4	NC	4
514		min	-.108	4	-.102	2	-.032	2	-1.001e-2	3	2414.722	2	7587.109	2
515	11	max	.003	1	.094	3	.026	3	1.431e-2	2	NC	5	NC	1
516		min	-.108	4	-.168	2	-.018	2	-9.243e-3	3	1260.775	2	9913.147	3
517	12	max	.003	1	.156	3	.074	1	1.31e-2	2	NC	5	NC	5
518		min	-.108	4	-.313	2	-.006	10	-8.473e-3	3	614.474	2	2198.374	1
519	13	max	.003	1	.223	3	.142	1	1.188e-2	2	NC	5	NC	5
520		min	-.108	4	-.472	2	.003	10	-7.703e-3	3	394.057	2	1190.117	1
521	14	max	.003	1	.274	3	.186	1	1.066e-2	2	NC	15	NC	5
522		min	-.108	4	-.593	2	.006	15	-6.933e-3	3	308.904	2	911.728	1
523	15	max	.003	1	.295	3	.196	1	9.449e-3	2	NC	15	NC	5
524		min	-.108	4	-.644	2	.001	15	-6.163e-3	3	283.158	2	868.849	1
525	16	max	.003	1	.277	3	.168	1	8.233e-3	2	NC	15	NC	3
526		min	-.108	4	-.607	2	-.005	5	-5.393e-3	3	301.329	2	1010.246	1
527	17	max	.003	1	.219	3	.11	1	7.017e-3	2	NC	5	NC	3
528		min	-.108	4	-.48	2	-.01	5	-4.624e-3	3	386.31	2	1511.487	1
529	18	max	.003	1	.13	3	.043	1	5.801e-3	2	NC	5	NC	3
530		min	-.108	4	-.278	2	-.01	5	-3.854e-3	3	702.212	2	3635.447	1
531	19	max	.003	1	.02	3	.008	3	4.585e-3	2	NC	1	NC	1
532		min	-.108	4	-.03	2	-.009	2	-3.084e-3	3	NC	1	NC	1
533	M15	max	0	1	0	1	0	1	3.763e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.739e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.012	4	9.013e-4	3	NC	5	NC	1
536		min	0	5	-.014	1	0	3	-7.036e-4	5	6848.567	2	8092.488	4
537	3	max	0	3	0	15	.026	4	1.426e-3	3	NC	5	NC	1
538		min	-.002	5	-.028	1	-.004	3	-1.19e-3	2	3485.002	2	3674.853	4
539	4	max	0	3	-.001	15	.041	4	1.951e-3	3	NC	5	NC	9
540		min	-.003	5	-.04	1	-.008	3	-1.751e-3	2	2390.915	2	2337.507	4
541	5	max	0	3	-.002	15	.055	4	2.476e-3	3	NC	5	NC	9
542		min	-.004	5	-.052	1	-.013	3	-2.312e-3	2	1865.655	2	1737.397	4
543	6	max	0	3	-.002	15	.067	4	3.001e-3	3	NC	5	9147.337	9
544		min	-.004	5	-.061	1	-.018	3	-2.873e-3	2	1570.146	2	1422.18	4
545	7	max	0	3	-.002	15	.076	4	3.527e-3	3	NC	5	7223.068	9
546		min	-.005	5	-.069	1	-.024	3	-3.434e-3	2	1392.436	2	1248.397	4
547	8	max	0	3	-.002	15	.082	4	4.052e-3	3	NC	5	6001.849	9
548		min	-.006	5	-.075	1	-.029	3	-3.994e-3	2	1285.784	2	1159.413	4
549	9	max	0	3	-.002	15	.084	4	4.577e-3	3	NC	5	5197.897	9
550		min	-.007	5	-.079	1	-.034	3	-4.555e-3	2	1228.377	2	1131.675	4
551	10	max	0	3	-.002	15	.082	4	5.102e-3	3	NC	5	4666.138	9
552		min	-.008	5	-.08	1	-.038	3	-5.116e-3	2	1210.216	2	1158.346	4
553	11	max	0	3	-.002	15	.076	4	5.627e-3	3	NC	5	4330.076	9
554		min	-.009	5	-.079	1	-.041	3	-5.677e-3	2	1228.377	2	1245.182	4
555	12	max	0	3	-.001	15	.067	4	6.152e-3	3	NC	5	4152.165	9

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556			min	-0.01	5	-0.075	1	-0.042	3	-6.238e-3	2	1285.784	2	1413.537	4
557		13	max	0	3	0	15	.055	4	6.677e-3	3	NC	5	4122.858	9
558			min	-0.011	5	-0.07	1	-.041	3	-6.799e-3	2	1392.436	2	1536.206	3
559		14	max	0	3	0	15	.041	4	7.202e-3	3	NC	5	4261.978	9
560			min	-0.012	5	-0.062	1	-.037	3	-7.36e-3	2	1570.146	2	1584.508	3
561		15	max	.001	3	0	15	.031	1	7.727e-3	3	NC	5	6873.205	15
562			min	-0.012	5	-0.053	1	-.03	3	-7.921e-3	2	1865.655	2	1720.714	3
563		16	max	.001	3	.002	5	.022	1	8.252e-3	3	NC	5	NC	5
564			min	-0.013	5	-0.041	1	-.02	3	-8.481e-3	2	2390.915	2	2011.769	3
565		17	max	.001	3	.003	5	.009	1	8.777e-3	3	NC	5	NC	4
566			min	-0.014	5	-0.029	1	-.007	3	-9.042e-3	2	3485.002	2	2667.641	3
567		18	max	.001	3	.005	5	.011	3	9.302e-3	3	NC	5	NC	4
568			min	-0.015	5	-0.016	1	-.014	2	-9.603e-3	2	6848.567	2	4750.412	3
569		19	max	.001	3	.007	5	.032	3	9.827e-3	3	NC	1	NC	1
570			min	-0.016	5	-0.003	9	-0.035	2	-1.016e-2	2	NC	1	NC	1
571	M16A	1	max	0	10	0	10	.01	3	2.909e-3	3	NC	1	NC	1
572			min	-0.006	4	-0.004	4	-.01	2	-2.816e-3	2	NC	1	NC	1
573		2	max	0	10	-0.007	12	.004	1	2.792e-3	3	NC	12	NC	2
574			min	-0.006	4	-0.027	4	-.004	5	-2.693e-3	2	4148.238	4	9565.214	1
575		3	max	0	10	-0.014	12	.012	1	2.674e-3	3	6970.004	12	NC	4
576			min	-0.005	4	-0.049	4	-.013	5	-2.57e-3	2	2110.897	4	5408.571	1
577		4	max	0	10	-.02	12	.018	1	2.556e-3	3	4781.83	12	NC	10
578			min	-0.005	4	-0.069	4	-.025	5	-2.447e-3	2	1448.199	4	4004.063	5
579		5	max	0	10	-0.026	12	.022	1	2.438e-3	3	3731.31	12	NC	10
580			min	-0.005	4	-0.088	4	-.04	5	-2.323e-3	2	1130.044	4	2458.578	5
581		6	max	0	10	-0.031	12	.025	1	2.32e-3	3	3140.292	12	9525.286	10
582			min	-0.004	4	-.103	4	-.056	5	-2.2e-3	2	951.052	4	1759.305	5
583		7	max	0	10	-0.034	12	.025	1	2.203e-3	3	2784.873	12	9398.971	10
584			min	-0.004	4	-.116	4	-.07	5	-2.077e-3	2	843.411	4	1389.66	5
585		8	max	0	10	-0.037	12	.025	1	2.085e-3	3	2571.567	12	9689.833	10
586			min	-0.004	4	-.125	4	-.082	5	-1.954e-3	2	778.811	4	1179.306	5
587		9	max	0	10	-0.039	12	.024	1	1.967e-3	3	2456.754	12	NC	10
588			min	-0.003	4	-.131	4	-.091	5	-1.831e-3	2	744.039	4	1059.446	5
589		10	max	0	10	-.04	12	.022	1	1.849e-3	3	2420.432	12	NC	10
590			min	-0.003	4	-.132	4	-.096	5	-1.708e-3	2	733.039	4	999.403	5
591		11	max	0	10	-0.039	12	.019	1	1.732e-3	3	2456.754	12	NC	10
592			min	-0.003	4	-.13	4	-.098	5	-1.585e-3	2	744.039	4	986.138	5
593		12	max	0	10	-0.037	12	.016	1	1.614e-3	3	2571.567	12	NC	9
594			min	-0.002	4	-.124	4	-.095	5	-1.461e-3	2	778.811	4	1017.265	5
595		13	max	0	10	-0.034	12	.012	1	1.496e-3	3	2784.873	12	NC	3
596			min	-0.002	4	-.115	4	-.087	5	-1.338e-3	2	843.411	4	1100.047	5
597		14	max	0	10	-.03	12	.009	1	1.378e-3	3	3140.292	12	NC	2
598			min	-0.002	4	-.102	4	-.077	5	-1.215e-3	2	951.052	4	1255.572	5
599		15	max	0	10	-0.026	12	.006	1	1.26e-3	3	3731.31	12	NC	1
600			min	-0.001	4	-.085	4	-.063	5	-1.092e-3	2	1130.044	4	1532.979	5
601		16	max	0	10	-.02	12	.003	1	1.143e-3	3	4781.83	12	NC	1
602			min	-0.001	4	-.067	4	-.047	5	-9.689e-4	2	1448.199	4	2055.242	5
603		17	max	0	10	-0.014	12	.001	9	1.025e-3	3	6970.004	12	NC	1
604			min	0	4	-.046	4	-.03	5	-8.458e-4	2	2110.897	4	3206.937	5
605		18	max	0	10	-.007	12	0	3	1.02e-3	4	NC	12	NC	1
606			min	0	4	-.023	4	-.014	5	-7.227e-4	2	4148.238	4	6965.676	5
607		19	max	0	1	0	1	0	1	1.094e-3	4	NC	1	NC	1
608			min	0	1	0	1	0	1	-5.995e-4	2	NC	1	NC	1



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

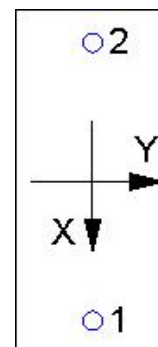
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.