

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.701 k
$M_y \text{ allowable}$ =	0.423 k-ft
$M_z \text{ allowable}$ =	0.423 k-ft
$P_n \text{ allowable}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.587 k
$M_y \text{ allowable}$ =	0.404 k-ft
$M_z \text{ allowable}$ =	0.404 k-ft
$P_n \text{ allowable}$ =	3.814 k
Utilization =	15%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.608 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.134 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 36 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

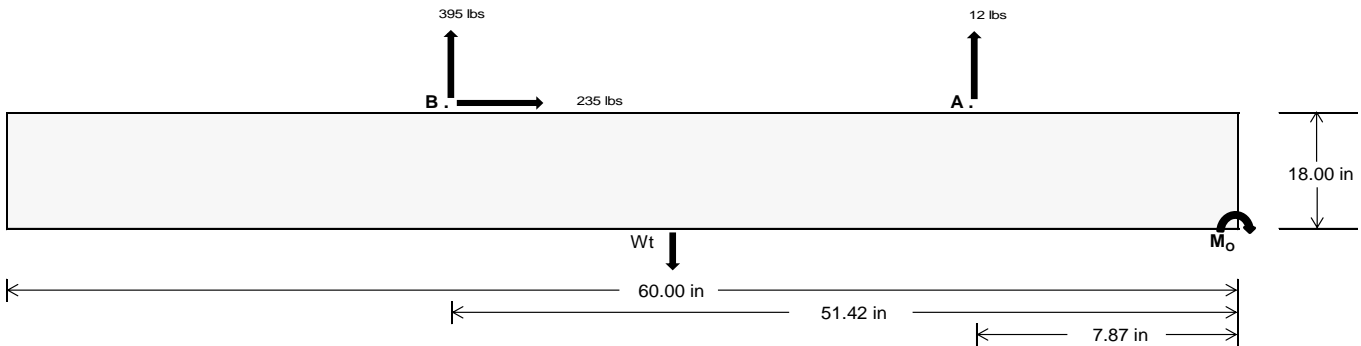
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>58.42</u>	<u>1713.73</u>	k
Compressive Load =	<u>911.11</u>	<u>1100.84</u>	k
Lateral Load =	<u>1.57</u>	<u>1019.73</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24634.9$ in-lbs
Resisting Force Required = 821.16 lbs
S.F. = 1.67
Weight Required = 1368.61 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 235.26 lbs
Friction = 0.4
Weight Required = 588.16 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding.
Friction is OK.

Cohesion

Sliding Force = 235.26 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	284 lbs	284 lbs	284 lbs	284 lbs	366 lbs	366 lbs	366 lbs	366 lbs	461 lbs	461 lbs	461 lbs	461 lbs	-25 lbs	-25 lbs	-25 lbs	-25 lbs
F_B	188 lbs	188 lbs	188 lbs	188 lbs	473 lbs	473 lbs	473 lbs	473 lbs	477 lbs	477 lbs	477 lbs	477 lbs	-790 lbs	-790 lbs	-790 lbs	-790 lbs
F_V	22 lbs	22 lbs	22 lbs	22 lbs	419 lbs	419 lbs	419 lbs	419 lbs	329 lbs	329 lbs	329 lbs	329 lbs	-471 lbs	-471 lbs	-471 lbs	-471 lbs
P_{total}	2284 lbs	2375 lbs	2465 lbs	2556 lbs	2652 lbs	2742 lbs	2833 lbs	2924 lbs	2751 lbs	2841 lbs	2932 lbs	3023 lbs	273 lbs	327 lbs	382 lbs	436 lbs
M	221 lbs-ft	221 lbs-ft	221 lbs-ft	221 lbs-ft	460 lbs-ft	460 lbs-ft	460 lbs-ft	460 lbs-ft	492 lbs-ft	492 lbs-ft	492 lbs-ft	492 lbs-ft	658 lbs-ft	658 lbs-ft	658 lbs-ft	658 lbs-ft
e	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.41 ft	2.01 ft	1.72 ft	1.51 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	242.3 psf	241.1 psf	240.0 psf	239.0 psf	251.9 psf	250.3 psf	248.8 psf	247.4 psf	259.2 psf	257.2 psf	255.4 psf	253.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	305.9 psf	301.7 psf	297.9 psf	294.4 psf	384.5 psf	376.6 psf	369.3 psf	362.7 psf	401.0 psf	392.3 psf	384.3 psf	377.1 psf	1215.8 psf	254.4 psf	178.8 psf	153.0 psf

Maximum Bearing Pressure = 1216 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

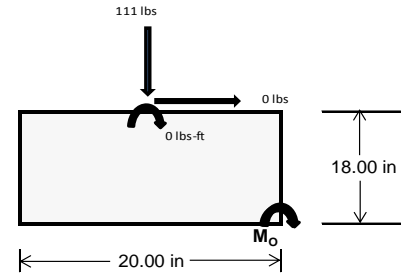
Overturning Check

$M_o = 0.0 \text{ ft-lbs}$
 Resisting Force Required = 0.00 lbs
 S.F. = 1.67
 Weight Required = 0.00 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	48 lbs	111 lbs	45 lbs	144 lbs	392 lbs	141 lbs	14 lbs	33 lbs	13 lbs
F_v	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2292 lbs	2355 lbs	2289 lbs	2280 lbs	2528 lbs	2277 lbs	670 lbs	689 lbs	669 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
f_{min}	274.9 sqft	282.6 sqft	274.6 sqft	273.3 sqft	303.3 sqft	273.1 sqft	80.4 sqft	82.6 sqft	80.3 sqft
f_{max}	275.1 psf	282.7 psf	274.7 psf	273.8 psf	303.5 psf	273.4 psf	80.4 psf	82.6 psf	80.3 psf



Maximum Bearing Pressure = 304 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.873 k
Allowable Uplift =	1.214 k
Utilization =	<u>72%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.095 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.701 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>

Diagonal Strut

Maximum Axial Load =	0.587 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>



Rear Strut

Maximum Axial Load =	1.068 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.134 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.003 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.9$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 23.9$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 30$$

$$Cc = 30$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 30.0 \text{ ksi}$$

$$I_x = 250988 \text{ mm}^4$$

$$0.603 \text{ in}^4$$

$$y = 30 \text{ mm}$$

$$S_x = 0.511 \text{ in}^3$$

$$M_{maxSt} = 1.276 \text{ k-ft}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20$$

$$Cc = 20$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.5 \text{ ksi}$$

$$I_y = 120291 \text{ mm}^4$$

$$0.289 \text{ in}^4$$

$$x = 20 \text{ mm}$$

$$S_y = 0.367 \text{ in}^3$$

$$M_{maxWk} = 0.871 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 7.4$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

$$b/t = 23.9$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.47 \text{ ksi}$$

$$A = 578.06 \text{ mm}^2$$

$$0.90 \text{ in}^2$$

$$P_{max} = 25.51 \text{ kips}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &21.4323 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.32 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} F_{cy})}{D_c} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - D_c * L_b / (1.2 * r_y * \sqrt{(C_b)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = Fut + (Fst - Fut)\rho_{st} < Fst$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{maxSt} = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{maxWk} = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$R_b/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 0.77182 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83792 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 24.5226 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 24.52 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 12.31 \text{ kips}\end{aligned}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (Cb \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (Cb \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.404 \text{ k-ft}$$

$$\phi F_{LWk} = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.68476$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.81587$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 10.0603 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 10.06 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{max} = 5.05 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL								

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-45.999	-45.999	0	0
2	M16	Y	-45.999	-45.999	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	-128.904	-128.904	0	0
2	M16	y	-207.368	-207.368	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	257.809	257.809	0	0
2	M16	y	123.3	123.3	0	0

Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.5W	Yes	Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes	Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes	Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								
8																				
9	ASD 1.0D + 1.0S	Yes	Y		1	1	3	1												
10	ASD 1.0D + 0.6W	Yes	Y		1	1			4	.6										
11	ASD 1.0D + 0.75L + 0.45W + 0....	Yes	Y		1	1	3	.75	4	.45										
12	ASD 0.6D + 0.6W	Yes	Y		2	.6					5	.6								
13	LATERAL - ASD 1.238D + 0.875E	Yes	Y		1	1.2...					6	.875								
14	LATERAL - ASD 1.1785D + 0.65...	Yes	Y		1	1.1...	3	.75			6	.656								
15	LATERAL - ASD 0.362D + 0.875E	Yes	Y		1	.362					6	.875								



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	235.754	2	281.067	2	.006	10	0	10	0	1	0	1
2		min	-274.298	3	-429.03	3	-.165	3	0	3	0	1	0	1
3	N7	max	.002	3	246.199	1	.057	10	0	10	0	1	0	1
4		min	-.125	2	3.643	12	-.432	1	0	1	0	1	0	1
5	N15	max	0	15	700.85	2	.081	9	0	9	0	1	0	1
6		min	-1.205	2	-44.94	3	-.75	3	-.001	3	0	1	0	1
7	N16	max	706.901	2	846.801	2	0	2	0	9	0	1	0	1
8		min	-784.405	3	-1318.253	3	-96.325	3	0	3	0	1	0	1
9	N23	max	.002	3	246.553	1	.464	3	0	3	0	1	0	1
10		min	-.126	2	4.14	12	-.056	10	0	10	0	1	0	1
11	N24	max	235.754	2	283.306	2	97.205	3	0	9	0	1	0	1
12		min	-275.117	3	-428.755	3	-.006	10	0	3	0	1	0	1
13	Totals:	max	1176.952	2	2558.404	2	0	1						
14		min	-1333.881	3	-2211.29	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	198.209	2	.656	4	.064	1	0	10	0	10	0	1
2			min	-366.522	3	.154	15	-.079	3	0	3	0	1	0	1
3		2	max	198.335	2	.605	4	.064	1	0	10	0	10	0	15
4			min	-366.428	3	.142	15	-.079	3	0	3	0	3	0	4
5		3	max	198.461	2	.554	4	.064	1	0	10	0	15	0	15
6			min	-366.334	3	.13	15	-.079	3	0	3	0	3	0	4
7		4	max	198.587	2	.503	4	.064	1	0	10	0	9	0	15
8			min	-366.239	3	.118	15	-.079	3	0	3	0	3	0	4
9		5	max	198.713	2	.451	4	.064	1	0	10	0	9	0	15
10			min	-366.145	3	.106	15	-.079	3	0	3	0	3	0	4
11		6	max	198.839	2	.4	4	.064	1	0	10	0	9	0	15
12			min	-366.05	3	.094	15	-.079	3	0	3	0	3	0	4
13		7	max	198.964	2	.349	4	.064	1	0	10	0	9	0	15
14			min	-365.956	3	.082	15	-.079	3	0	3	0	3	0	4
15		8	max	199.09	2	.298	4	.064	1	0	10	0	9	0	15
16			min	-365.862	3	.07	15	-.079	3	0	3	0	3	0	4
17		9	max	199.216	2	.247	4	.064	1	0	10	0	9	0	15
18			min	-365.767	3	.058	15	-.079	3	0	3	0	3	0	4
19		10	max	199.342	2	.196	4	.064	1	0	10	0	9	0	15
20			min	-365.673	3	.046	15	-.079	3	0	3	0	3	0	4
21		11	max	199.468	2	.145	4	.064	1	0	10	0	9	0	15
22			min	-365.578	3	.033	12	-.079	3	0	3	0	3	0	4
23		12	max	199.594	2	.105	2	.064	1	0	10	0	9	0	15
24			min	-365.484	3	.013	12	-.079	3	0	3	0	3	0	4
25		13	max	199.72	2	.065	2	.064	1	0	10	0	9	0	15
26			min	-365.39	3	-.015	3	-.079	3	0	3	0	3	0	4
27		14	max	199.845	2	.025	2	.064	1	0	10	0	9	0	15
28			min	-365.295	3	-.045	3	-.079	3	0	3	0	3	0	4
29		15	max	199.971	2	-.014	15	.064	1	0	10	0	9	0	15
30			min	-365.201	3	-.074	3	-.079	3	0	3	0	3	0	4
31		16	max	200.097	2	-.026	15	.064	1	0	10	0	9	0	15
32			min	-365.106	3	-.111	4	-.079	3	0	3	0	3	0	4
33		17	max	200.223	2	-.038	15	.064	1	0	10	0	9	0	15
34			min	-365.012	3	-.162	4	-.079	3	0	3	0	3	0	4
35		18	max	200.349	2	-.05	15	.064	1	0	10	0	9	0	15
36			min	-364.918	3	-.213	4	-.079	3	0	3	0	3	0	4
37		19	max	200.475	2	-.062	15	.064	1	0	10	0	9	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-364.823	3	-.264	4	-.079	3	0	3	0	3	0	4
39	M3	1	max	197.13	2	1.759	4	.011	10	0	10	0	1	4
40		min	-182.966	3	.414	15	-.099	1	0	1	0	10	0	15
41		2	max	197.06	2	1.582	4	.011	10	0	10	0	1	2
42		min	-183.018	3	.372	15	-.099	1	0	1	0	10	0	12
43		3	max	196.991	2	1.405	4	.011	10	0	10	0	1	2
44		min	-183.07	3	.33	15	-.099	1	0	1	0	10	0	3
45		4	max	196.922	2	1.229	4	.011	10	0	10	0	1	15
46		min	-183.122	3	.289	15	-.099	1	0	1	0	10	0	4
47		5	max	196.852	2	1.052	4	.011	10	0	10	0	1	15
48		min	-183.174	3	.247	15	-.099	1	0	1	0	10	0	4
49		6	max	196.783	2	.875	4	.011	10	0	10	0	1	15
50		min	-183.226	3	.206	15	-.099	1	0	1	0	10	0	4
51		7	max	196.714	2	.698	4	.011	10	0	10	0	1	15
52		min	-183.278	3	.164	15	-.099	1	0	1	0	10	0	4
53		8	max	196.644	2	.521	4	.011	10	0	10	0	1	15
54		min	-183.33	3	.123	15	-.099	1	0	1	0	10	-.001	4
55		9	max	196.575	2	.344	4	.011	10	0	10	0	1	15
56		min	-183.382	3	.081	15	-.099	1	0	1	0	10	-.001	4
57		10	max	196.506	2	.168	4	.011	10	0	10	0	1	15
58		min	-183.434	3	.04	15	-.099	1	0	1	0	10	-.001	4
59		11	max	196.436	2	.02	2	.011	10	0	10	0	1	15
60		min	-183.486	3	-.039	3	-.099	1	0	1	0	10	-.001	4
61		12	max	196.367	2	-.044	15	.011	10	0	10	0	1	15
62		min	-183.538	3	-.186	4	-.099	1	0	1	0	10	-.001	4
63		13	max	196.298	2	-.085	15	.011	10	0	10	0	1	15
64		min	-183.59	3	-.363	4	-.099	1	0	1	0	10	-.001	4
65		14	max	196.228	2	-.127	15	.011	10	0	10	0	1	15
66		min	-183.642	3	-.54	4	-.099	1	0	1	0	10	-.001	4
67		15	max	196.159	2	-.168	15	.011	10	0	10	0	9	15
68		min	-183.694	3	-.717	4	-.099	1	0	1	0	10	0	4
69		16	max	196.09	2	-.21	15	.011	10	0	10	0	9	15
70		min	-183.746	3	-.893	4	-.099	1	0	1	0	1	0	4
71		17	max	196.02	2	-.251	15	.011	10	0	10	0	10	15
72		min	-183.798	3	-1.07	4	-.099	1	0	1	0	1	0	4
73		18	max	195.951	2	-.293	15	.011	10	0	10	0	10	15
74		min	-183.85	3	-1.247	4	-.099	1	0	1	0	1	0	4
75		19	max	195.882	2	-.335	15	.011	10	0	10	0	10	1
76		min	-183.902	3	-1.424	4	-.099	1	0	1	0	1	0	1
77	M4	1	max	245.034	1	0	1	.058	10	0	1	0	3	1
78		min	3.061	12	0	1	-.45	1	0	1	0	2	0	1
79		2	max	245.099	1	0	1	.058	10	0	1	0	10	1
80		min	3.093	12	0	1	-.45	1	0	1	0	1	0	1
81		3	max	245.163	1	0	1	.058	10	0	1	0	10	1
82		min	3.125	12	0	1	-.45	1	0	1	0	1	0	1
83		4	max	245.228	1	0	1	.058	10	0	1	0	10	1
84		min	3.158	12	0	1	-.45	1	0	1	0	1	0	1
85		5	max	245.293	1	0	1	.058	10	0	1	0	10	1
86		min	3.19	12	0	1	-.45	1	0	1	0	1	0	1
87		6	max	245.357	1	0	1	.058	10	0	1	0	10	1
88		min	3.223	12	0	1	-.45	1	0	1	0	1	0	1
89		7	max	245.422	1	0	1	.058	10	0	1	0	10	1
90		min	3.255	12	0	1	-.45	1	0	1	0	1	0	1
91		8	max	245.487	1	0	1	.058	10	0	1	0	10	1
92		min	3.287	12	0	1	-.45	1	0	1	0	1	0	1
93		9	max	245.552	1	0	1	.058	10	0	1	0	10	1
94		min	3.32	12	0	1	-.45	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	245.616	1	0	1	.058	10	0	1	0	10	0	1
96		min	3.352	12	0	1	-.45	1	0	1	0	1	0	1
97	11	max	245.681	1	0	1	.058	10	0	1	0	10	0	1
98		min	3.384	12	0	1	-.45	1	0	1	0	1	0	1
99	12	max	245.746	1	0	1	.058	10	0	1	0	10	0	1
100		min	3.417	12	0	1	-.45	1	0	1	0	1	0	1
101	13	max	245.81	1	0	1	.058	10	0	1	0	10	0	1
102		min	3.449	12	0	1	-.45	1	0	1	0	1	0	1
103	14	max	245.875	1	0	1	.058	10	0	1	0	10	0	1
104		min	3.481	12	0	1	-.45	1	0	1	0	1	0	1
105	15	max	245.94	1	0	1	.058	10	0	1	0	10	0	1
106		min	3.514	12	0	1	-.45	1	0	1	0	1	0	1
107	16	max	246.005	1	0	1	.058	10	0	1	0	10	0	1
108		min	3.546	12	0	1	-.45	1	0	1	0	1	0	1
109	17	max	246.069	1	0	1	.058	10	0	1	0	10	0	1
110		min	3.578	12	0	1	-.45	1	0	1	0	1	0	1
111	18	max	246.134	1	0	1	.058	10	0	1	0	10	0	1
112		min	3.611	12	0	1	-.45	1	0	1	0	1	0	1
113	19	max	246.199	1	0	1	.058	10	0	1	0	10	0	1
114		min	3.643	12	0	1	-.45	1	0	1	0	1	0	1
115	M6	1	max	605.858	2	.655	.011	9	0	3	0	3	0	1
116		min	-1067.959	3	.154	15	-.305	3	0	1	0	1	0	1
117	2	max	605.984	2	.604	4	.011	9	0	3	0	3	0	15
118		min	-1067.864	3	.142	15	-.305	3	0	1	0	1	0	4
119	3	max	606.11	2	.553	4	.011	9	0	3	0	3	0	15
120		min	-1067.77	3	.13	15	-.305	3	0	1	0	1	0	4
121	4	max	606.236	2	.502	4	.011	9	0	3	0	3	0	15
122		min	-1067.676	3	.118	15	-.305	3	0	1	0	1	0	4
123	5	max	606.362	2	.451	4	.011	9	0	3	0	3	0	15
124		min	-1067.581	3	.106	15	-.305	3	0	1	0	1	0	4
125	6	max	606.487	2	.404	2	.011	9	0	3	0	3	0	15
126		min	-1067.487	3	.087	12	-.305	3	0	1	0	1	0	4
127	7	max	606.613	2	.365	2	.011	9	0	3	0	9	0	15
128		min	-1067.392	3	.067	12	-.305	3	0	1	0	1	0	4
129	8	max	606.739	2	.325	2	.011	9	0	3	0	9	0	15
130		min	-1067.298	3	.047	12	-.305	3	0	1	0	3	0	4
131	9	max	606.865	2	.285	2	.011	9	0	3	0	9	0	15
132		min	-1067.204	3	.027	12	-.305	3	0	1	0	3	0	4
133	10	max	606.991	2	.245	2	.011	9	0	3	0	9	0	12
134		min	-1067.109	3	0	3	-.305	3	0	1	0	3	0	4
135	11	max	607.117	2	.205	2	.011	9	0	3	0	9	0	12
136		min	-1067.015	3	-.031	3	-.305	3	0	1	0	3	0	2
137	12	max	607.243	2	.165	2	.011	9	0	3	0	9	0	12
138		min	-1066.92	3	-.06	3	-.305	3	0	1	0	3	0	2
139	13	max	607.369	2	.126	2	.011	9	0	3	0	9	0	12
140		min	-1066.826	3	-.09	3	-.305	3	0	1	0	3	0	2
141	14	max	607.494	2	.086	2	.011	9	0	3	0	9	0	12
142		min	-1066.732	3	-.12	3	-.305	3	0	1	0	3	0	2
143	15	max	607.62	2	.046	2	.011	9	0	3	0	9	0	12
144		min	-1066.637	3	-.15	3	-.305	3	0	1	0	3	0	2
145	16	max	607.746	2	.006	2	.011	9	0	3	0	9	0	12
146		min	-1066.543	3	-.18	3	-.305	3	0	1	0	3	0	2
147	17	max	607.872	2	-.034	2	.011	9	0	3	0	9	0	12
148		min	-1066.448	3	-.21	3	-.305	3	0	1	0	3	0	2
149	18	max	607.998	2	-.05	15	.011	9	0	3	0	9	0	3
150		min	-1066.354	3	-.24	3	-.305	3	0	1	0	3	0	2
151	19	max	608.124	2	-.062	15	.011	9	0	3	0	9	0	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1066.26	3	-.27	3	-.305	3	0	1	0	3	0	2
153		max	587.349	2	1.761	4	.042	3	0	9	0	9	0	2
154		min	-485.767	3	.414	15	-.003	9	0	3	0	3	0	3
155		max	587.28	2	1.584	4	.042	3	0	9	0	9	0	2
156		min	-485.819	3	.372	15	-.003	9	0	3	0	3	0	3
157		max	587.21	2	1.407	4	.042	3	0	9	0	9	0	2
158		min	-485.871	3	.331	15	-.003	9	0	3	0	3	0	3
159		max	587.141	2	1.23	4	.042	3	0	9	0	9	0	2
160		min	-485.923	3	.289	15	-.003	9	0	3	0	3	0	3
161		max	587.072	2	1.054	4	.042	3	0	9	0	9	0	15
162		min	-485.975	3	.248	15	-.003	9	0	3	0	3	0	3
163		max	587.002	2	.877	4	.042	3	0	9	0	9	0	15
164		min	-486.027	3	.206	15	-.003	9	0	3	0	3	0	4
165		max	586.933	2	.7	4	.042	3	0	9	0	9	0	15
166		min	-486.079	3	.165	15	-.003	9	0	3	0	3	0	4
167		max	586.864	2	.523	4	.042	3	0	9	0	9	0	15
168		min	-486.131	3	.123	15	-.003	9	0	3	0	3	-.001	4
169		max	586.794	2	.348	2	.042	3	0	9	0	9	0	15
170	M8	min	-486.183	3	.081	12	-.003	9	0	3	0	3	-.001	4
171		max	586.725	2	.21	2	.042	3	0	9	0	9	0	15
172		min	-486.235	3	.009	3	-.003	9	0	3	0	3	-.001	4
173		max	586.656	2	.072	2	.042	3	0	9	0	9	0	15
174		min	-486.287	3	-.095	3	-.003	9	0	3	0	3	-.001	4
175		max	586.587	2	-.043	15	.042	3	0	9	0	9	0	15
176		min	-486.339	3	-.198	3	-.003	9	0	3	0	3	-.001	4
177		max	586.517	2	-.085	15	.042	3	0	9	0	9	0	15
178		min	-486.391	3	-.361	4	-.003	9	0	3	0	3	-.001	4
179		max	586.448	2	-.126	15	.042	3	0	9	0	9	0	15
180		min	-486.443	3	-.538	4	-.003	9	0	3	0	3	-.001	4
181		max	586.379	2	-.168	15	.042	3	0	9	0	9	0	15
182		min	-486.495	3	-.715	4	-.003	9	0	3	0	3	0	4
183		max	586.309	2	-.21	15	.042	3	0	9	0	9	0	15
184		min	-486.547	3	-.892	4	-.003	9	0	3	0	3	0	4
185		max	586.24	2	-.251	15	.042	3	0	9	0	9	0	15
186		min	-486.599	3	-1.068	4	-.003	9	0	3	0	3	0	4
187		max	586.171	2	-.293	15	.042	3	0	9	0	9	0	15
188		min	-486.651	3	-1.245	4	-.003	9	0	3	0	3	0	4
189	M8	max	586.101	2	-.334	15	.042	3	0	9	0	9	0	1
190		min	-486.703	3	-1.422	4	-.003	9	0	3	0	3	0	1
191		max	699.686	2	0	1	.085	9	0	1	0	1	0	1
192		min	-45.814	3	0	1	-.744	3	0	1	0	3	0	1
193		max	699.75	2	0	1	.085	9	0	1	0	9	0	1
194		min	-45.765	3	0	1	-.744	3	0	1	0	3	0	1
195		max	699.815	2	0	1	.085	9	0	1	0	9	0	1
196		min	-45.717	3	0	1	-.744	3	0	1	0	3	0	1
197		max	699.88	2	0	1	.085	9	0	1	0	9	0	1
198		min	-45.668	3	0	1	-.744	3	0	1	0	3	0	1
199		max	699.944	2	0	1	.085	9	0	1	0	9	0	1
200		min	-45.62	3	0	1	-.744	3	0	1	0	3	0	1
201		max	700.009	2	0	1	.085	9	0	1	0	9	0	1
202		min	-45.571	3	0	1	-.744	3	0	1	0	3	0	1
203		max	700.074	2	0	1	.085	9	0	1	0	9	0	1
204		min	-45.523	3	0	1	-.744	3	0	1	0	3	0	1
205		max	700.139	2	0	1	.085	9	0	1	0	9	0	1
206		min	-45.474	3	0	1	-.744	3	0	1	0	3	0	1
207		max	700.203	2	0	1	.085	9	0	1	0	9	0	1
208		min	-45.425	3	0	1	-.744	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	700.268	2	0	1	.085	9	0	1	0	9	0	1
210			min	-45.377	3	0	1	-.744	3	0	1	0	3	0	1
211		11	max	700.333	2	0	1	.085	9	0	1	0	9	0	1
212			min	-45.328	3	0	1	-.744	3	0	1	0	3	0	1
213		12	max	700.397	2	0	1	.085	9	0	1	0	9	0	1
214			min	-45.28	3	0	1	-.744	3	0	1	0	3	0	1
215		13	max	700.462	2	0	1	.085	9	0	1	0	9	0	1
216			min	-45.231	3	0	1	-.744	3	0	1	0	3	0	1
217		14	max	700.527	2	0	1	.085	9	0	1	0	9	0	1
218			min	-45.183	3	0	1	-.744	3	0	1	0	3	0	1
219		15	max	700.592	2	0	1	.085	9	0	1	0	9	0	1
220			min	-45.134	3	0	1	-.744	3	0	1	0	3	0	1
221		16	max	700.656	2	0	1	.085	9	0	1	0	9	0	1
222			min	-45.086	3	0	1	-.744	3	0	1	0	3	0	1
223		17	max	700.721	2	0	1	.085	9	0	1	0	9	0	1
224			min	-45.037	3	0	1	-.744	3	0	1	-.001	3	0	1
225		18	max	700.786	2	0	1	.085	9	0	1	0	9	0	1
226			min	-44.989	3	0	1	-.744	3	0	1	-.001	3	0	1
227		19	max	700.85	2	0	1	.085	9	0	1	0	9	0	1
228			min	-44.94	3	0	1	-.744	3	0	1	-.001	3	0	1
229	M10	1	max	199.384	2	.656	4	.005	10	0	1	0	9	0	1
230			min	-275.478	3	.154	15	-.065	1	0	3	0	3	0	1
231		2	max	199.51	2	.605	4	.005	10	0	1	0	9	0	15
232			min	-275.384	3	.142	15	-.065	1	0	3	0	3	0	4
233		3	max	199.636	2	.554	4	.005	10	0	1	0	9	0	15
234			min	-275.289	3	.13	15	-.065	1	0	3	0	3	0	4
235		4	max	199.762	2	.503	4	.005	10	0	1	0	9	0	15
236			min	-275.195	3	.118	15	-.065	1	0	3	0	3	0	4
237		5	max	199.888	2	.451	4	.005	10	0	1	0	9	0	15
238			min	-275.1	3	.106	15	-.065	1	0	3	0	3	0	4
239		6	max	200.014	2	.4	4	.005	10	0	1	0	9	0	15
240			min	-275.006	3	.094	15	-.065	1	0	3	0	3	0	4
241		7	max	200.14	2	.349	4	.005	10	0	1	0	9	0	15
242			min	-274.912	3	.082	15	-.065	1	0	3	0	3	0	4
243		8	max	200.265	2	.298	4	.005	10	0	1	0	10	0	15
244			min	-274.817	3	.07	15	-.065	1	0	3	0	3	0	4
245		9	max	200.391	2	.247	4	.005	10	0	1	0	10	0	15
246			min	-274.723	3	.058	15	-.065	1	0	3	0	3	0	4
247		10	max	200.517	2	.196	4	.005	10	0	1	0	10	0	15
248			min	-274.628	3	.046	15	-.065	1	0	3	0	3	0	4
249		11	max	200.643	2	.145	4	.005	10	0	1	0	10	0	15
250			min	-274.534	3	.034	15	-.065	1	0	3	0	3	0	4
251		12	max	200.769	2	.105	2	.005	10	0	1	0	10	0	15
252			min	-274.44	3	.018	12	-.065	1	0	3	0	3	0	4
253		13	max	200.895	2	.065	2	.005	10	0	1	0	10	0	15
254			min	-274.345	3	-.006	3	-.065	1	0	3	0	3	0	4
255		14	max	201.021	2	.025	2	.005	10	0	1	0	10	0	15
256			min	-274.251	3	-.036	3	-.065	1	0	3	0	3	0	4
257		15	max	201.147	2	-.014	15	.005	10	0	1	0	10	0	15
258			min	-274.156	3	-.065	3	-.065	1	0	3	0	3	0	4
259		16	max	201.272	2	-.026	15	.005	10	0	1	0	10	0	15
260			min	-274.062	3	-.111	4	-.065	1	0	3	0	3	0	4
261		17	max	201.398	2	-.038	15	.005	10	0	1	0	10	0	15
262			min	-273.968	3	-.162	4	-.065	1	0	3	0	3	0	4
263		18	max	201.524	2	-.05	15	.005	10	0	1	0	10	0	15
264			min	-273.873	3	-.213	4	-.065	1	0	3	0	3	0	4
265		19	max	201.65	2	-.062	15	.005	10	0	1	0	10	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-273.779	3	-.265	4	-.065	1	0	3	0	3	0	4
267	M11	1	max	196.733	2	1.759	4	.099	1	0	3	0	3	0	4
268			min	-183.933	3	.414	15	-.061	3	0	10	0	1	0	15
269		2	max	196.664	2	1.582	4	.099	1	0	3	0	3	0	2
270			min	-183.985	3	.372	15	-.061	3	0	10	0	1	0	12
271		3	max	196.595	2	1.405	4	.099	1	0	3	0	3	0	2
272			min	-184.037	3	.33	15	-.061	3	0	10	0	1	0	3
273		4	max	196.525	2	1.229	4	.099	1	0	3	0	3	0	15
274			min	-184.089	3	.289	15	-.061	3	0	10	0	1	0	4
275		5	max	196.456	2	1.052	4	.099	1	0	3	0	3	0	15
276			min	-184.141	3	.247	15	-.061	3	0	10	0	1	0	4
277		6	max	196.387	2	.875	4	.099	1	0	3	0	3	0	15
278			min	-184.193	3	.206	15	-.061	3	0	10	0	1	0	4
279		7	max	196.317	2	.698	4	.099	1	0	3	0	3	0	15
280			min	-184.245	3	.164	15	-.061	3	0	10	0	1	0	4
281		8	max	196.248	2	.521	4	.099	1	0	3	0	3	0	15
282			min	-184.297	3	.123	15	-.061	3	0	10	0	1	-.001	4
283		9	max	196.179	2	.344	4	.099	1	0	3	0	3	0	15
284			min	-184.349	3	.081	15	-.061	3	0	10	0	1	-.001	4
285		10	max	196.109	2	.168	4	.099	1	0	3	0	3	0	15
286			min	-184.401	3	.039	15	-.061	3	0	10	0	1	-.001	4
287		11	max	196.04	2	.02	2	.099	1	0	3	0	3	0	15
288			min	-184.453	3	-.04	3	-.061	3	0	10	0	1	-.001	4
289		12	max	195.971	2	-.044	15	.099	1	0	3	0	3	0	15
290			min	-184.505	3	-.186	4	-.061	3	0	10	0	1	-.001	4
291		13	max	195.901	2	-.085	15	.099	1	0	3	0	3	0	15
292			min	-184.557	3	-.363	4	-.061	3	0	10	0	1	-.001	4
293		14	max	195.832	2	-.127	15	.099	1	0	3	0	3	0	15
294			min	-184.609	3	-.54	4	-.061	3	0	10	0	1	-.001	4
295		15	max	195.763	2	-.168	15	.099	1	0	3	0	3	0	15
296			min	-184.661	3	-.717	4	-.061	3	0	10	0	1	0	4
297		16	max	195.693	2	-.21	15	.099	1	0	3	0	3	0	15
298			min	-184.713	3	-.893	4	-.061	3	0	10	0	10	0	4
299		17	max	195.624	2	-.251	15	.099	1	0	3	0	3	0	15
300			min	-184.765	3	-1.07	4	-.061	3	0	10	0	10	0	4
301		18	max	195.555	2	-.293	15	.099	1	0	3	0	3	0	15
302			min	-184.817	3	-1.247	4	-.061	3	0	10	0	10	0	4
303		19	max	195.485	2	-.335	15	.099	1	0	3	0	3	0	1
304			min	-184.869	3	-1.424	4	-.061	3	0	10	0	10	0	1
305	M12	1	max	245.388	1	0	1	.464	3	0	1	0	2	0	1
306			min	3.558	12	0	1	-.057	10	0	1	0	3	0	1
307		2	max	245.453	1	0	1	.464	3	0	1	0	1	0	1
308			min	3.59	12	0	1	-.057	10	0	1	0	10	0	1
309		3	max	245.517	1	0	1	.464	3	0	1	0	1	0	1
310			min	3.623	12	0	1	-.057	10	0	1	0	10	0	1
311		4	max	245.582	1	0	1	.464	3	0	1	0	1	0	1
312			min	3.655	12	0	1	-.057	10	0	1	0	10	0	1
313		5	max	245.647	1	0	1	.464	3	0	1	0	1	0	1
314			min	3.687	12	0	1	-.057	10	0	1	0	10	0	1
315		6	max	245.711	1	0	1	.464	3	0	1	0	1	0	1
316			min	3.72	12	0	1	-.057	10	0	1	0	10	0	1
317		7	max	245.776	1	0	1	.464	3	0	1	0	1	0	1
318			min	3.752	12	0	1	-.057	10	0	1	0	10	0	1
319		8	max	245.841	1	0	1	.464	3	0	1	0	1	0	1
320			min	3.784	12	0	1	-.057	10	0	1	0	10	0	1
321		9	max	245.906	1	0	1	.464	3	0	1	0	1	0	1
322			min	3.817	12	0	1	-.057	10	0	1	0	10	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-64.641	1	-166.426	3	-12.658	1	0	2	-.024	1	0	3
381	M5	1	max	167.963	1	1082.992	3	0	1	0	.013	3	0	3
382		min	-8.229	3	-673.427	2	-87.508	3	0	3	0	11	0	2
383		2	max	168.103	1	1082.811	3	0	1	0	0	9	.145	2
384		min	-8.124	3	-673.669	2	-87.508	3	0	3	-.006	3	-.234	3
385		3	max	241.647	3	4.636	9	9.297	3	0	0	9	.289	2
386		min	-51.479	2	-82.542	2	-.098	9	0	1	-.024	3	-.464	3
387		4	max	241.751	3	4.434	9	9.297	3	0	0	9	.307	2
388		min	-51.34	2	-82.784	2	-.098	9	0	1	-.022	3	-.453	3
389		5	max	241.856	3	4.233	9	9.297	3	0	0	9	.325	2
390		min	-51.2	2	-83.026	2	-.098	9	0	1	-.02	3	-.442	3
391		6	max	241.961	3	4.031	9	9.297	3	0	0	9	.343	2
392		min	-51.06	2	-83.268	2	-.098	9	0	1	-.018	3	-.431	3
393		7	max	242.066	3	3.829	9	9.297	3	0	0	9	.361	2
394		min	-50.921	2	-83.51	2	-.098	9	0	1	-.015	3	-.42	3
395		8	max	242.17	3	3.628	9	9.297	3	0	0	9	.379	2
396		min	-50.781	2	-83.752	2	-.098	9	0	1	-.013	3	-.409	3
397		9	max	242.275	3	3.426	9	9.297	3	0	0	9	.398	2
398		min	-50.641	2	-83.993	2	-.098	9	0	1	-.011	3	-.398	3
399		10	max	242.38	3	3.225	9	9.297	3	0	0	1	.416	2
400		min	-50.502	2	-84.235	2	-.098	9	0	1	-.009	3	-.387	3
401		11	max	242.484	3	3.023	9	9.297	3	0	0	1	.434	2
402		min	-50.362	2	-84.477	2	-.098	9	0	1	-.007	3	-.376	3
403		12	max	242.589	3	2.822	9	9.297	3	0	0	1	.453	2
404		min	-50.223	2	-84.719	2	-.098	9	0	1	-.005	3	-.365	3
405		13	max	242.694	3	2.62	9	9.297	3	0	0	1	.471	2
406		min	-50.083	2	-84.961	2	-.098	9	0	1	-.003	3	-.353	3
407		14	max	242.799	3	2.419	9	9.297	3	0	0	1	.489	2
408		min	-49.943	2	-85.202	2	-.098	9	0	1	-.001	3	-.342	3
409		15	max	242.903	3	2.217	9	9.297	3	0	0	3	.508	2
410		min	-49.804	2	-85.444	2	-.098	9	0	1	0	9	-.331	3
411		16	max	272.632	2	406.713	2	9.27	3	0	.002	3	.522	2
412		min	-23.61	3	-459.599	3	-.1	9	0	1	0	9	-.316	3
413		17	max	272.772	2	406.471	2	9.27	3	0	.004	3	.434	2
414		min	-23.506	3	-459.781	3	-.1	9	0	1	0	9	-.216	3
415		18	max	-.55	3	1007.682	2	8.528	3	0	.006	3	.218	2
416		min	-168.114	1	-499.862	3	-.018	9	0	9	0	9	-.108	3
417		19	max	-.446	3	1007.44	2	8.528	3	0	.008	3	0	3
418		min	-167.974	1	-500.043	3	-.018	9	0	9	0	9	0	2
419	M9	1	max	64.647	1	345.701	3	92.914	3	0	.003	10	0	2
420		min	2.726	15	-218.968	2	-1.378	10	0	2	-.024	1	0	3
421		2	max	64.786	1	345.52	3	92.914	3	0	.002	10	.048	2
422		min	2.768	15	-219.21	2	-1.378	10	0	2	-.021	1	-.075	3
423		3	max	91.965	3	3.802	9	12.112	1	0	.017	3	.095	2
424		min	-20.369	2	-25.686	2	-2.945	3	0	10	-.018	1	-.149	3
425		4	max	92.07	3	3.6	9	12.112	1	0	.016	3	.1	2
426		min	-20.229	2	-25.928	2	-2.945	3	0	10	-.016	1	-.146	3
427		5	max	92.175	3	3.398	9	12.112	1	0	.015	3	.106	2
428		min	-20.089	2	-26.17	2	-2.945	3	0	10	-.013	1	-.143	3
429		6	max	92.279	3	3.197	9	12.112	1	0	.015	3	.112	2
430		min	-19.95	2	-26.412	2	-2.945	3	0	10	-.011	1	-.14	3
431		7	max	92.384	3	2.995	9	12.112	1	0	.014	3	.117	2
432		min	-19.81	2	-26.654	2	-2.945	3	0	10	-.008	1	-.138	3
433		8	max	92.489	3	2.794	9	12.112	1	0	.013	3	.123	2
434		min	-19.67	2	-26.895	2	-2.945	3	0	10	-.005	1	-.135	3
435		9	max	92.594	3	2.592	9	12.112	1	0	.013	3	.129	2
436		min	-19.531	2	-27.137	2	-2.945	3	0	10	-.003	1	-.132	3





RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 30° 160mph 30psf 3.5ft 7-10 NPS 30



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.167	3	0	1	0	3	0	1
552		min	-133.397	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.167	3	0	1	0	3	0	1
554		min	-133.468	3	-.082	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.167	3	0	1	0	3	0	1
556		min	-133.538	3	-.164	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.167	3	0	1	0	3	0	1
558		min	-133.609	3	-.246	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.167	3	0	1	0	3	0	1
560		min	-133.679	3	-.327	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.167	3	0	1	0	3	0	1
562		min	-133.75	3	-.409	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.167	3	0	1	0	3	0	1
564		min	-133.82	3	-.491	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.167	3	0	1	0	3	0	1
566		min	-133.891	3	-.573	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.167	3	0	1	0	3	0	1
568		min	-133.961	3	-.655	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.167	3	0	1	0	3	0	1
570		min	-134.032	3	-.737	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.261	.012	9	0	3	0	3	0	1
572		min	-132.073	3	0	1	-.07	3	0	9	0	9	0	1
573	2	max	0	1	1.12	4	.012	9	0	3	0	3	0	1
574		min	-132.003	3	0	1	-.07	3	0	9	0	9	0	4
575	3	max	0	1	.98	4	.012	9	0	3	0	3	0	1
576		min	-131.932	3	0	1	-.07	3	0	9	0	9	0	4
577	4	max	0	1	.84	4	.012	9	0	3	0	3	0	1
578		min	-131.862	3	0	1	-.07	3	0	9	0	9	0	4
579	5	max	0	1	.7	4	.012	9	0	3	0	3	0	1
580		min	-131.791	3	0	1	-.07	3	0	9	0	9	-.001	4
581	6	max	0	1	.56	4	.012	9	0	3	0	3	0	1
582		min	-131.721	3	0	1	-.07	3	0	9	0	9	-.001	4
583	7	max	0	1	.42	4	.012	9	0	3	0	3	0	1
584		min	-131.65	3	0	1	-.07	3	0	9	0	9	-.001	4
585	8	max	0	1	.28	4	.012	9	0	3	0	3	0	1
586		min	-131.58	3	0	1	-.07	3	0	9	0	9	-.001	4
587	9	max	0	1	.14	4	.012	9	0	3	0	3	0	1
588		min	-131.509	3	0	1	-.07	3	0	9	0	9	-.001	4
589	10	max	0	1	0	1	.012	9	0	3	0	3	0	1
590		min	-131.439	3	0	1	-.07	3	0	9	0	9	-.002	4
591	11	max	.039	13	0	1	.012	9	0	3	0	3	0	1
592		min	-131.368	3	-.14	4	-.07	3	0	9	0	9	-.001	4
593	12	max	.136	13	0	1	.012	9	0	3	0	3	0	1
594		min	-131.298	3	-.28	4	-.07	3	0	9	0	14	-.001	4
595	13	max	.233	13	0	1	.012	9	0	3	0	1	0	1
596		min	-131.227	3	-.42	4	-.07	3	0	9	0	4	-.001	4
597	14	max	.341	4	0	1	.012	9	0	3	0	9	0	1
598		min	-131.157	3	-.56	4	-.07	3	0	9	0	3	-.001	4
599	15	max	.461	4	0	1	.012	9	0	3	0	9	0	1
600		min	-131.086	3	-.7	4	-.07	3	0	9	0	3	-.001	4
601	16	max	.582	4	0	1	.012	9	0	3	0	9	0	1
602		min	-131.016	3	-.84	4	-.07	3	0	9	0	3	0	4
603	17	max	.703	4	0	1	.012	9	0	3	0	9	0	1
604		min	-130.945	3	-.98	4	-.07	3	0	9	0	3	0	4
605	18	max	.823	4	0	1	.012	9	0	3	0	9	0	1
606		min	-130.875	3	-1.12	4	-.07	3	0	9	0	3	0	4
607	19	max	.944	4	0	1	.012	9	0	3	0	9	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-130.804	3	-1.261	4	-0.07	3	0	9	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.01	2	.002	9	2.265e-5	10	NC	3	NC	1	
2			min	-.004	3	-.01	3	-.003	3	-2.216e-4	3	4108.037	2	NC	1	
3			2	max	.002	2	.009	2	.002	9	2.158e-5	10	NC	3	NC	1
4				min	-.003	3	-.009	3	-.002	3	-2.098e-4	3	4481.036	2	NC	1
5			3	max	.002	2	.008	2	.001	9	2.05e-5	10	NC	3	NC	1
6				min	-.003	3	-.009	3	-.002	3	-1.98e-4	3	4924.187	2	NC	1
7			4	max	.002	2	.007	2	.001	9	1.943e-5	10	NC	1	NC	1
8				min	-.003	3	-.008	3	-.002	3	-1.861e-4	3	5454.303	2	NC	1
9			5	max	.002	2	.006	2	.001	9	1.836e-5	10	NC	1	NC	1
10				min	-.003	3	-.008	3	-.002	3	-1.743e-4	3	6093.759	2	NC	1
11		6	max	.001	2	.006	2	.001	9	1.728e-5	10	NC	1	NC	1	
12			min	-.003	3	-.008	3	-.001	3	-1.625e-4	3	6872.814	2	NC	1	
13		7	max	.001	2	.005	2	0	9	1.621e-5	10	NC	1	NC	1	
14			min	-.002	3	-.007	3	-.001	3	-1.507e-4	3	7833.166	2	NC	1	
15		8	max	.001	2	.004	2	0	9	1.514e-5	10	NC	1	NC	1	
16			min	-.002	3	-.007	3	-.001	3	-1.389e-4	3	9033.549	2	NC	1	
17		9	max	.001	2	.004	2	0	9	1.406e-5	10	NC	1	NC	1	
18			min	-.002	3	-.006	3	0	3	-1.27e-4	3	NC	1	NC	1	
19		10	max	0	2	.003	2	0	9	1.299e-5	10	NC	1	NC	1	
20			min	-.002	3	-.006	3	0	3	-1.152e-4	3	NC	1	NC	1	
21		11	max	0	2	.003	2	0	9	1.192e-5	10	NC	1	NC	1	
22			min	-.002	3	-.005	3	0	3	-1.034e-4	3	NC	1	NC	1	
23		12	max	0	2	.002	2	0	9	1.084e-5	10	NC	1	NC	1	
24			min	-.001	3	-.005	3	0	3	-9.401e-5	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	9	9.768e-6	10	NC	1	NC	1	
26			min	-.001	3	-.004	3	0	3	-8.474e-5	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	9	8.695e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-7.547e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	9	7.621e-6	10	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-6.62e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	9	6.548e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-5.693e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	9	5.474e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-4.765e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	9	4.401e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-3.838e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	3.327e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.038e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.436e-5	9	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.579e-6	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	1.982e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	9	-2.234e-6	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	2.583e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	9	-2.888e-6	10	NC	1	NC	1
45			4	max	0	3	0	2	0	3	3.183e-5	1	NC	1	NC	1
46				min	0	2	-.003	3	0	9	-3.542e-6	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	3.784e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	9	-4.197e-6	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	4.385e-5	1	NC	1	NC	1
50				min	0	2	-.004	3	0	9	-4.851e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	4.985e-5	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	-5.505e-6	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	5.586e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	10	-6.159e-6	10	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	6.186e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-6.814e-6	10	NC	1	NC	1
57		10	max	.001	3	.002	2	0	3	6.787e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-7.468e-6	10	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	7.387e-5	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-8.122e-6	10	NC	1	NC	1
61		12	max	.001	3	.003	2	0	1	7.988e-5	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-8.776e-6	10	NC	1	NC	1
63		13	max	.001	3	.004	2	0	1	8.589e-5	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-9.431e-6	10	NC	1	NC	1
65		14	max	.002	3	.004	2	0	1	9.189e-5	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-1.009e-5	10	NC	1	NC	1
67		15	max	.002	3	.005	2	.001	1	9.79e-5	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-1.074e-5	10	8893.752	2	NC	1
69		16	max	.002	3	.006	2	.001	1	1.039e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-1.139e-5	10	7543.653	2	NC	1
71		17	max	.002	3	.007	2	.001	1	1.099e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-1.205e-5	10	6497.074	2	NC	1
73		18	max	.002	3	.008	2	.002	1	1.159e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-1.27e-5	10	5676.903	2	NC	1
75		19	max	.002	3	.009	2	.002	1	1.219e-4	1	NC	3	NC	1
76			min	-.002	2	-.009	3	0	10	-1.336e-5	10	5028.646	2	NC	1
77	M4	1	max	.001	1	.011	2	0	10	1.575e-5	10	NC	1	NC	1
78			min	0	12	-.01	3	-.001	1	-1.396e-4	1	NC	1	NC	1
79		2	max	.001	1	.01	2	0	10	1.575e-5	10	NC	1	NC	1
80			min	0	12	-.009	3	-.001	1	-1.396e-4	1	NC	1	NC	1
81		3	max	.001	1	.01	2	0	10	1.575e-5	10	NC	1	NC	1
82			min	0	12	-.009	3	-.001	1	-1.396e-4	1	NC	1	NC	1
83		4	max	0	1	.009	2	0	10	1.575e-5	10	NC	1	NC	1
84			min	0	12	-.008	3	-.001	1	-1.396e-4	1	NC	1	NC	1
85		5	max	0	1	.008	2	0	10	1.575e-5	10	NC	1	NC	1
86			min	0	12	-.007	3	0	1	-1.396e-4	1	NC	1	NC	1
87		6	max	0	1	.008	2	0	10	1.575e-5	10	NC	1	NC	1
88			min	0	12	-.007	3	0	1	-1.396e-4	1	NC	1	NC	1
89		7	max	0	1	.007	2	0	10	1.575e-5	10	NC	1	NC	1
90			min	0	12	-.006	3	0	1	-1.396e-4	1	NC	1	NC	1
91		8	max	0	1	.007	2	0	10	1.575e-5	10	NC	1	NC	1
92			min	0	12	-.006	3	0	1	-1.396e-4	1	NC	1	NC	1
93		9	max	0	1	.006	2	0	10	1.575e-5	10	NC	1	NC	1
94			min	0	12	-.005	3	0	1	-1.396e-4	1	NC	1	NC	1
95		10	max	0	1	.005	2	0	10	1.575e-5	10	NC	1	NC	1
96			min	0	12	-.005	3	0	1	-1.396e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	10	1.575e-5	10	NC	1	NC	1
98			min	0	12	-.004	3	0	1	-1.396e-4	1	NC	1	NC	1
99		12	max	0	1	.004	2	0	10	1.575e-5	10	NC	1	NC	1
100			min	0	12	-.004	3	0	1	-1.396e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	10	1.575e-5	10	NC	1	NC	1
102			min	0	12	-.003	3	0	1	-1.396e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	10	1.575e-5	10	NC	1	NC	1
104			min	0	12	-.003	3	0	1	-1.396e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	10	1.575e-5	10	NC	1	NC	1
106			min	0	12	-.002	3	0	1	-1.396e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	10	1.575e-5	10	NC	1	NC	1
108			min	0	12	-.002	3	0	1	-1.396e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	1.575e-5	10	NC	1	NC	1
110			min	0	12	-.001	3	0	1	-1.396e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	1.575e-5	10	NC	1	NC	1
112			min	0	12	0	3	0	1	-1.396e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	1.575e-5	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.396e-4	1	NC	1	NC	1
115	M6	1	max	.006	2	.029	2	0	9	5.157e-4	3	NC	3	NC	1
116			min	-.01	3	-.028	3	-.007	3	-9.045e-8	1	1350.35	2	5671.195	3
117		2	max	.006	2	.027	2	0	9	5.e-4	3	NC	3	NC	1
118			min	-.01	3	-.026	3	-.007	3	-2.062e-7	9	1446.044	2	6014.061	3
119		3	max	.005	2	.025	2	0	9	4.844e-4	3	NC	3	NC	1
120			min	-.009	3	-.025	3	-.006	3	-8.919e-7	9	1555.85	2	6422.858	3
121		4	max	.005	2	.023	2	0	9	4.687e-4	3	NC	3	NC	1
122			min	-.009	3	-.023	3	-.006	3	-1.578e-6	9	1682.605	2	6911.536	3
123		5	max	.005	2	.022	2	0	9	4.53e-4	3	NC	3	NC	1
124			min	-.008	3	-.022	3	-.005	3	-2.263e-6	9	1829.964	2	7498.659	3
125		6	max	.004	2	.02	2	0	9	4.373e-4	3	NC	3	NC	1
126			min	-.007	3	-.02	3	-.005	3	-2.949e-6	9	2002.714	2	8209.259	3
127		7	max	.004	2	.018	2	0	9	4.216e-4	3	NC	3	NC	1
128			min	-.007	3	-.019	3	-.004	3	-3.635e-6	9	2207.243	2	9077.687	3
129		8	max	.004	2	.016	2	0	9	4.06e-4	3	NC	3	NC	1
130			min	-.006	3	-.017	3	-.004	3	-4.32e-6	9	2452.272	2	NC	1
131		9	max	.003	2	.014	2	0	9	3.903e-4	3	NC	3	NC	1
132			min	-.006	3	-.016	3	-.003	3	-5.006e-6	9	2750.006	2	NC	1
133		10	max	.003	2	.013	2	0	9	3.746e-4	3	NC	3	NC	1
134			min	-.005	3	-.014	3	-.003	3	-5.692e-6	9	3118.082	2	NC	1
135		11	max	.003	2	.011	2	0	9	3.589e-4	3	NC	3	NC	1
136			min	-.005	3	-.013	3	-.003	3	-6.377e-6	9	3582.949	2	NC	1
137		12	max	.002	2	.009	2	0	9	3.432e-4	3	NC	3	NC	1
138			min	-.004	3	-.011	3	-.002	3	-7.063e-6	9	4186.159	2	NC	1
139		13	max	.002	2	.008	2	0	9	3.276e-4	3	NC	3	NC	1
140			min	-.003	3	-.01	3	-.002	3	-7.749e-6	9	4996.949	2	NC	1
141		14	max	.002	2	.006	2	0	9	3.119e-4	3	NC	1	NC	1
142			min	-.003	3	-.008	3	-.001	3	-8.435e-6	9	6139.915	2	NC	1
143		15	max	.001	2	.005	2	0	9	2.962e-4	3	NC	1	NC	1
144			min	-.002	3	-.006	3	-.001	3	-9.12e-6	9	7864.187	2	NC	1
145		16	max	0	2	.004	2	0	9	2.805e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-9.806e-6	9	NC	1	NC	1
147		17	max	0	2	.002	2	0	9	2.648e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-1.049e-5	9	NC	1	NC	1
149		18	max	0	2	.001	2	0	9	2.492e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-1.118e-5	9	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.335e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.186e-5	9	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	5.562e-6	9	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.096e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	4.893e-6	9	NC	1	NC	1
156			min	0	2	-.002	3	0	9	-8.281e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	.001	3	4.224e-6	9	NC	1	NC	1
158			min	0	2	-.004	3	0	9	-5.606e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	3.555e-6	9	NC	1	NC	1
160			min	-.001	2	-.006	3	0	9	-2.932e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	2.885e-6	9	NC	1	NC	1
162			min	-.001	2	-.007	3	0	9	-2.576e-6	3	9342.157	2	NC	1
163		6	max	.002	3	.006	2	.002	3	2.417e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	9	0	2	7482.063	2	NC	1
165		7	max	.002	3	.007	2	.003	3	5.091e-5	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.011	3	0	9	0	5	6207.992	2	NC	1
167		8	max	.002	3	.009	2	.003	3	7.766e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	-5.327e-8	13	5272.694	2	NC	1
169		9	max	.002	3	.01	2	.003	3	1.044e-4	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	9	-1.41e-7	13	4553.012	2	NC	1
171		10	max	.003	3	.012	2	.003	3	1.311e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	9	-4.605e-7	9	3980.672	2	NC	1
173		11	max	.003	3	.013	2	.003	3	1.579e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	-1.13e-6	9	3514.633	2	NC	1
175		12	max	.003	3	.015	2	.003	3	1.846e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	9	-1.799e-6	9	3128.582	2	NC	1
177		13	max	.004	3	.016	2	.004	3	2.114e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	0	9	-2.468e-6	9	2804.718	2	NC	1
179		14	max	.004	3	.018	2	.004	3	2.381e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-3.137e-6	9	2530.451	2	NC	1
181		15	max	.004	3	.02	2	.004	3	2.649e-4	3	NC	3	NC	1
182			min	-.005	2	-.021	3	0	9	-3.806e-6	9	2296.533	2	NC	1
183		16	max	.005	3	.022	2	.004	3	2.916e-4	3	NC	3	NC	1
184			min	-.006	2	-.022	3	0	9	-4.476e-6	9	2095.969	2	NC	1
185		17	max	.005	3	.024	2	.004	3	3.184e-4	3	NC	3	NC	1
186			min	-.006	2	-.023	3	0	9	-5.145e-6	9	1923.333	2	NC	1
187		18	max	.005	3	.026	2	.003	3	3.451e-4	3	NC	3	NC	1
188			min	-.006	2	-.024	3	0	9	-5.814e-6	9	1774.341	2	NC	1
189		19	max	.006	3	.028	2	.003	3	3.718e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	9	-6.483e-6	9	1645.561	2	NC	1
191	M8	1	max	.003	2	.033	2	0	9	-1.161e-7	10	NC	1	NC	1
192			min	0	3	-.028	3	-.002	3	-2.688e-4	3	NC	1	8216.779	3
193		2	max	.003	2	.031	2	0	9	-1.161e-7	10	NC	1	NC	1
194			min	0	3	-.026	3	-.002	3	-2.688e-4	3	NC	1	8958.9	3
195		3	max	.003	2	.03	2	0	9	-1.161e-7	10	NC	1	NC	1
196			min	0	3	-.025	3	-.002	3	-2.688e-4	3	NC	1	9842.378	3
197		4	max	.003	2	.028	2	0	9	-1.161e-7	10	NC	1	NC	1
198			min	0	3	-.023	3	-.002	3	-2.688e-4	3	NC	1	NC	1
199		5	max	.003	2	.026	2	0	9	-1.161e-7	10	NC	1	NC	1
200			min	0	3	-.021	3	-.002	3	-2.688e-4	3	NC	1	NC	1
201		6	max	.002	2	.024	2	0	9	-1.161e-7	10	NC	1	NC	1
202			min	0	3	-.02	3	-.001	3	-2.688e-4	3	NC	1	NC	1
203		7	max	.002	2	.022	2	0	9	-1.161e-7	10	NC	1	NC	1
204			min	0	3	-.018	3	-.001	3	-2.688e-4	3	NC	1	NC	1
205		8	max	.002	2	.02	2	0	9	-1.161e-7	10	NC	1	NC	1
206			min	0	3	-.017	3	-.001	3	-2.688e-4	3	NC	1	NC	1
207		9	max	.002	2	.018	2	0	9	-1.161e-7	10	NC	1	NC	1
208			min	0	3	-.015	3	0	3	-2.688e-4	3	NC	1	NC	1
209		10	max	.002	2	.017	2	0	9	-1.161e-7	10	NC	1	NC	1
210			min	0	3	-.014	3	0	3	-2.688e-4	3	NC	1	NC	1
211		11	max	.001	2	.015	2	0	9	-1.161e-7	10	NC	1	NC	1
212			min	0	3	-.012	3	0	3	-2.688e-4	3	NC	1	NC	1
213		12	max	.001	2	.013	2	0	9	-1.161e-7	10	NC	1	NC	1
214			min	0	3	-.011	3	0	3	-2.688e-4	3	NC	1	NC	1
215		13	max	.001	2	.011	2	0	9	-1.161e-7	10	NC	1	NC	1
216			min	0	3	-.009	3	0	3	-2.688e-4	3	NC	1	NC	1
217		14	max	0	2	.009	2	0	9	-1.161e-7	10	NC	1	NC	1
218			min	0	3	-.008	3	0	3	-2.688e-4	3	NC	1	NC	1
219		15	max	0	2	.007	2	0	9	-1.161e-7	10	NC	1	NC	1
220			min	0	3	-.006	3	0	3	-2.688e-4	3	NC	1	NC	1
221		16	max	0	2	.006	2	0	9	-1.161e-7	10	NC	1	NC	1
222			min	0	3	-.005	3	0	3	-2.688e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	2	.004	2	0	9	-1.161e-7	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.688e-4	3	NC	1	NC	1
225		18	max	0	2	.002	2	0	9	-1.161e-7	10	NC	1	NC	1
226			min	0	3	-.002	3	0	3	-2.688e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.161e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.688e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.01	2	0	10	1.958e-4	1	NC	3	NC	1
230			min	-.003	3	-.01	3	-.001	1	-6.069e-4	3	4111.702	2	NC	1
231		2	max	.002	2	.009	2	0	10	1.866e-4	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.001	1	-5.859e-4	3	4485.145	2	NC	1
233		3	max	.002	2	.008	2	0	10	1.773e-4	1	NC	3	NC	1
234			min	-.002	3	-.009	3	-.001	1	-5.65e-4	3	4928.844	2	NC	1
235		4	max	.002	2	.007	2	0	3	1.68e-4	1	NC	1	NC	1
236			min	-.002	3	-.008	3	-.001	1	-5.441e-4	3	5459.644	2	NC	1
237		5	max	.002	2	.006	2	0	3	1.588e-4	1	NC	1	NC	1
238			min	-.002	3	-.008	3	-.001	1	-5.232e-4	3	6099.962	2	NC	1
239		6	max	.001	2	.006	2	0	3	1.495e-4	1	NC	1	NC	1
240			min	-.002	3	-.008	3	0	1	-5.023e-4	3	6880.116	2	NC	1
241		7	max	.001	2	.005	2	0	3	1.402e-4	1	NC	1	NC	1
242			min	-.002	3	-.007	3	0	1	-4.814e-4	3	7841.889	2	NC	1
243		8	max	.001	2	.004	2	0	3	1.31e-4	1	NC	1	NC	1
244			min	-.002	3	-.007	3	0	1	-4.605e-4	3	9044.142	2	NC	1
245		9	max	.001	2	.004	2	0	3	1.217e-4	1	NC	1	NC	1
246			min	-.001	3	-.006	3	0	1	-4.395e-4	3	NC	1	NC	1
247		10	max	0	2	.003	2	0	3	1.125e-4	1	NC	1	NC	1
248			min	-.001	3	-.006	3	0	1	-4.186e-4	3	NC	1	NC	1
249		11	max	0	2	.003	2	0	3	1.032e-4	1	NC	1	NC	1
250			min	-.001	3	-.005	3	0	1	-3.977e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	9.394e-5	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-3.768e-4	3	NC	1	NC	1
253		13	max	0	2	.002	2	0	3	8.468e-5	1	NC	1	NC	1
254			min	0	3	-.004	3	0	1	-3.559e-4	3	NC	1	NC	1
255		14	max	0	2	.001	2	0	3	7.542e-5	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-3.35e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	6.616e-5	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-3.141e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	5.69e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.931e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	4.764e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.722e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	3.838e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.513e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.912e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.304e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.088e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.382e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	8.297e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.981e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.717e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-2.58e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	1	3.137e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-3.18e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	1	5.566e-6	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-3.779e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	4.903e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-4.378e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	5.568e-6	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-4.977e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	6.232e-6	10	NC	1	NC	1
282			min	0	2	-.006	3	-.003	3	-7.184e-5	3	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	6.897e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-9.764e-5	3	NC	1	NC	1
285		10	max	.001	3	.002	2	0	10	7.562e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.234e-4	3	NC	1	NC	1
287		11	max	.001	3	.002	2	0	10	8.226e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-1.492e-4	3	NC	1	NC	1
289		12	max	.001	3	.003	2	0	10	8.891e-6	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.003	3	-1.75e-4	3	NC	1	NC	1
291		13	max	.001	3	.004	2	0	10	9.556e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-2.008e-4	3	NC	1	NC	1
293		14	max	.002	3	.004	2	0	10	1.022e-5	10	NC	1	NC	1
294			min	-.002	2	-.008	3	-.003	3	-2.266e-4	3	NC	1	NC	1
295		15	max	.002	3	.005	2	0	10	1.089e-5	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.003	3	-2.524e-4	3	8904.909	2	NC	1
297		16	max	.002	3	.006	2	0	10	1.155e-5	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	3	-2.782e-4	3	7552.214	2	NC	1
299		17	max	.002	3	.007	2	0	10	1.221e-5	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.003	3	-3.04e-4	3	6503.82	2	NC	1
301		18	max	.002	3	.008	2	0	10	1.288e-5	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.003	3	-3.298e-4	3	5682.354	2	NC	1
303		19	max	.002	3	.009	2	0	10	1.354e-5	10	NC	3	NC	1
304			min	-.002	2	-.009	3	-.002	3	-3.556e-4	3	5033.159	2	NC	1
305	M12	1	max	.001	1	.011	2	.001	1	3.913e-4	3	NC	1	NC	1
306			min	0	12	-.01	3	0	10	-1.598e-5	10	NC	1	NC	1
307		2	max	.001	1	.01	2	.001	1	3.913e-4	3	NC	1	NC	1
308			min	0	12	-.009	3	0	10	-1.598e-5	10	NC	1	NC	1
309		3	max	.001	1	.01	2	.001	3	3.913e-4	3	NC	1	NC	1
310			min	0	12	-.009	3	0	10	-1.598e-5	10	NC	1	NC	1
311		4	max	0	1	.009	2	.001	3	3.913e-4	3	NC	1	NC	1
312			min	0	12	-.008	3	0	10	-1.598e-5	10	NC	1	NC	1
313		5	max	0	1	.008	2	0	3	3.913e-4	3	NC	1	NC	1
314			min	0	12	-.008	3	0	10	-1.598e-5	10	NC	1	NC	1
315		6	max	0	1	.008	2	0	3	3.913e-4	3	NC	1	NC	1
316			min	0	12	-.007	3	0	10	-1.598e-5	10	NC	1	NC	1
317		7	max	0	1	.007	2	0	3	3.913e-4	3	NC	1	NC	1
318			min	0	12	-.006	3	0	10	-1.598e-5	10	NC	1	NC	1
319		8	max	0	1	.007	2	0	3	3.913e-4	3	NC	1	NC	1
320			min	0	12	-.006	3	0	10	-1.598e-5	10	NC	1	NC	1
321		9	max	0	1	.006	2	0	3	3.913e-4	3	NC	1	NC	1
322			min	0	12	-.005	3	0	10	-1.598e-5	10	NC	1	NC	1
323		10	max	0	1	.005	2	0	3	3.913e-4	3	NC	1	NC	1
324			min	0	12	-.005	3	0	10	-1.598e-5	10	NC	1	NC	1
325		11	max	0	1	.005	2	0	3	3.913e-4	3	NC	1	NC	1
326			min	0	12	-.004	3	0	10	-1.598e-5	10	NC	1	NC	1
327		12	max	0	1	.004	2	0	3	3.913e-4	3	NC	1	NC	1
328			min	0	12	-.004	3	0	10	-1.598e-5	10	NC	1	NC	1
329		13	max	0	1	.004	2	0	3	3.913e-4	3	NC	1	NC	1
330			min	0	12	-.003	3	0	10	-1.598e-5	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	3	3.913e-4	3	NC	1	NC	1
332			min	0	12	-.003	3	0	10	-1.598e-5	10	NC	1	NC	1
333		15	max	0	1	.002	2	0	3	3.913e-4	3	NC	1	NC	1
334			min	0	12	-.002	3	0	10	-1.598e-5	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	3	3.913e-4	3	NC	1	NC	1
336			min	0	12	-.002	3	0	10	-1.598e-5	10	NC	1	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	3	3.913e-4	3	NC	1	NC	1
338			min	0	12	-.001	3	0	10	-1.598e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	3	3.913e-4	3	NC	1	NC	1
340			min	0	12	0	3	0	10	-1.598e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.913e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.598e-5	10	NC	1	NC	1
343	M1	1	max	.009	3	.025	3	.004	3	4.89e-3	2	NC	1	NC	1
344			min	-.009	2	-.021	2	0	9	-7.219e-3	3	NC	1	NC	1
345		2	max	.009	3	.015	3	.003	3	2.42e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.001	9	-3.549e-3	3	4646.666	3	NC	1
347		3	max	.009	3	.005	3	.003	3	5.326e-5	3	NC	4	NC	1
348			min	-.009	2	-.004	2	-.002	9	-8.507e-5	1	2406.75	3	NC	1
349		4	max	.009	3	.003	2	.002	3	5.46e-5	3	NC	4	NC	1
350			min	-.009	2	-.003	3	-.002	9	-7.129e-5	9	1714.528	3	NC	1
351		5	max	.009	3	.01	2	.002	3	5.593e-5	3	NC	4	NC	1
352			min	-.009	2	-.009	3	-.002	9	-5.838e-5	9	1384.384	3	NC	1
353		6	max	.009	3	.015	2	.001	3	5.727e-5	3	NC	4	NC	1
354			min	-.009	2	-.015	3	-.002	9	-4.547e-5	9	1200.074	3	NC	1
355		7	max	.009	3	.019	2	.001	3	5.861e-5	3	NC	4	NC	1
356			min	-.009	2	-.019	3	-.002	9	-3.257e-5	9	1090.926	3	NC	1
357		8	max	.009	3	.022	2	.001	3	5.995e-5	3	NC	4	NC	1
358			min	-.009	2	-.022	3	-.001	9	-1.966e-5	9	1022.673	2	NC	1
359		9	max	.009	3	.025	2	.001	3	6.129e-5	3	NC	4	NC	1
360			min	-.009	2	-.024	3	0	9	-6.755e-6	9	970.361	2	NC	1
361		10	max	.008	3	.025	2	.001	3	6.262e-5	3	NC	4	NC	1
362			min	-.009	2	-.024	3	0	9	-2.078e-6	10	945.219	2	NC	1
363		11	max	.008	3	.025	2	.001	3	6.396e-5	3	NC	4	NC	1
364			min	-.009	2	-.023	3	0	10	-3.892e-6	10	944.374	2	NC	1
365		12	max	.008	3	.023	2	.001	3	6.53e-5	3	NC	4	NC	1
366			min	-.009	2	-.021	3	0	10	-5.706e-6	10	968.792	2	NC	1
367		13	max	.008	3	.02	2	.001	1	7.017e-5	1	NC	4	NC	1
368			min	-.009	2	-.018	3	0	10	-7.52e-6	10	1023.846	2	NC	1
369		14	max	.008	3	.016	2	.002	1	8.569e-5	1	NC	4	NC	1
370			min	-.009	2	-.014	3	0	10	-9.333e-6	10	1122.084	2	NC	1
371		15	max	.008	3	.01	2	.002	1	1.012e-4	1	NC	4	NC	1
372			min	-.009	2	-.009	3	0	10	-1.115e-5	10	1291.189	2	NC	1
373		16	max	.008	3	.003	2	.002	1	1.127e-4	1	NC	4	NC	1
374			min	-.009	2	-.003	3	0	10	-1.249e-5	10	1599.223	2	NC	1
375		17	max	.008	3	.005	3	.001	1	8.187e-5	3	NC	4	NC	1
376			min	-.009	2	-.006	2	0	10	-2.691e-6	10	2266.745	2	NC	1
377		18	max	.008	3	.013	3	.001	3	3.511e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	10	-1.937e-3	3	4394.162	2	NC	1
379		19	max	.008	3	.021	3	0	3	7.086e-3	2	NC	1	NC	1
380			min	-.009	2	-.028	2	0	9	-3.995e-3	3	NC	1	NC	1
381	M5	1	max	.025	3	.076	3	.004	3	1.231e-5	3	NC	1	NC	1
382			min	-.027	2	-.063	2	0	9	0	15	NC	1	NC	1
383		2	max	.025	3	.045	3	.006	3	1.485e-4	3	NC	4	NC	1
384			min	-.027	2	-.037	2	0	9	-8.196e-6	9	1525.317	3	NC	1
385		3	max	.025	3	.016	3	.007	3	2.82e-4	3	NC	5	NC	1
386			min	-.027	2	-.012	2	0	9	-1.633e-5	9	790.617	3	NC	1
387		4	max	.025	3	.011	2	.008	3	2.722e-4	3	NC	5	NC	1
388			min	-.027	2	-.009	3	0	9	-1.548e-5	9	564.175	3	NC	1
389		5	max	.025	3	.03	2	.009	3	2.625e-4	3	NC	5	NC	1
390			min	-.027	2	-.029	3	0	9	-1.464e-5	9	456.347	3	8773.676	3
391		6	max	.024	3	.046	2	.009	3	2.528e-4	3	NC	5	NC	1
392			min	-.027	2	-.046	3	0	9	-1.379e-5	9	396.303	3	7929.802	3
393		7	max	.024	3	.059	2	.009	3	2.43e-4	3	NC	5	NC	1



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 30° 160mph 30psf 3.5ft 7-10 NPa and 39



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.008	3	.005	3	0	10	9.564e-5	3	NC	4	NC	1
452		min	-.009	2	-.006	2	-.001	3	-4.405e-5	9	2267.351	2	NC	1
453	18	max	.008	3	.013	3	0	10	2.027e-3	3	NC	4	NC	1
454		min	-.009	2	-.017	2	0	9	-3.512e-3	2	4395.299	2	NC	1
455	19	max	.008	3	.021	3	0	3	3.991e-3	3	NC	1	NC	1
456		min	-.009	2	-.028	2	0	9	-7.086e-3	2	NC	1	NC	1
457	M13	1	max	0	.024	3	.009	3	3.882e-3	3	NC	1	NC	1
458		min	-.004	3	-.021	2	-.009	2	-3.269e-3	2	NC	1	NC	1
459	2	max	0	9	.057	3	.007	3	4.775e-3	3	NC	4	NC	1
460		min	-.004	3	-.043	2	-.009	2	-4.021e-3	2	2567.378	3	NC	1
461	3	max	0	9	.085	3	.007	3	5.669e-3	3	NC	4	NC	1
462		min	-.004	3	-.062	2	-.009	2	-4.773e-3	2	1385.349	3	NC	1
463	4	max	0	9	.105	3	.008	3	6.562e-3	3	NC	4	NC	1
464		min	-.004	3	-.076	2	-.01	2	-5.526e-3	2	1044.098	3	NC	1
465	5	max	0	9	.115	3	.01	3	7.456e-3	3	NC	4	NC	1
466		min	-.004	3	-.084	2	-.013	2	-6.278e-3	2	928.004	3	NC	1
467	6	max	0	9	.115	3	.013	3	8.349e-3	3	NC	4	NC	1
468		min	-.004	3	-.085	2	-.016	2	-7.03e-3	2	924.946	3	NC	1
469	7	max	0	9	.108	3	.016	3	9.243e-3	3	NC	4	NC	1
470		min	-.004	3	-.081	2	-.019	2	-7.783e-3	2	1012.286	3	8167.43	2
471	8	max	0	9	.095	3	.019	3	1.014e-2	3	NC	4	NC	1
472		min	-.004	3	-.074	2	-.023	2	-8.535e-3	2	1195.178	3	6000.252	2
473	9	max	0	9	.082	3	.022	3	1.103e-2	3	NC	4	NC	4
474		min	-.004	3	-.067	2	-.026	2	-9.287e-3	2	1457.401	3	4942.683	2
475	10	max	0	9	.076	3	.025	3	1.192e-2	3	NC	4	NC	4
476		min	-.004	3	-.063	2	-.027	2	-1.004e-2	2	1627.869	3	4599.226	2
477	11	max	0	9	.083	3	.026	3	1.103e-2	3	NC	4	NC	4
478		min	-.004	3	-.067	2	-.026	2	-9.287e-3	2	1457.399	3	4760.791	3
479	12	max	0	9	.095	3	.027	3	1.014e-2	3	NC	4	NC	1
480		min	-.004	3	-.074	2	-.023	2	-8.535e-3	2	1195.177	3	4711.238	3
481	13	max	0	9	.108	3	.025	3	9.252e-3	3	NC	4	NC	1
482		min	-.004	3	-.081	2	-.019	2	-7.783e-3	2	1012.285	3	5013.384	3
483	14	max	0	9	.116	3	.023	3	8.362e-3	3	NC	4	NC	1
484		min	-.004	3	-.085	2	-.016	2	-7.03e-3	2	924.945	3	5727.208	3
485	15	max	0	9	.116	3	.021	3	7.471e-3	3	NC	4	NC	1
486		min	-.004	3	-.084	2	-.013	2	-6.278e-3	2	928.003	3	7091.706	3
487	16	max	0	9	.106	3	.017	3	6.581e-3	3	NC	4	NC	1
488		min	-.004	3	-.076	2	-.01	2	-5.526e-3	2	1044.097	3	9764.185	3
489	17	max	0	9	.086	3	.014	3	5.69e-3	3	NC	4	NC	1
490		min	-.004	3	-.062	2	-.009	2	-4.774e-3	2	1385.347	3	NC	1
491	18	max	0	9	.058	3	.011	3	4.8e-3	3	NC	4	NC	1
492		min	-.004	3	-.043	2	-.009	2	-4.021e-3	2	2567.376	3	NC	1
493	19	max	0	9	.025	3	.009	3	3.91e-3	3	NC	1	NC	1
494		min	-.004	3	-.021	2	-.009	2	-3.269e-3	2	NC	1	NC	1
495	M16	1	max	0	.021	3	.008	3	4.192e-3	2	NC	1	NC	1
496		min	0	3	-.028	2	-.009	2	-3.201e-3	3	NC	1	NC	1
497	2	max	0	9	.039	3	.011	3	5.159e-3	2	NC	4	NC	1
498		min	0	3	-.06	2	-.009	2	-3.893e-3	3	2616.303	2	NC	1
499	3	max	0	9	.055	3	.014	3	6.126e-3	2	NC	4	NC	1
500		min	0	3	-.087	2	-.009	2	-4.585e-3	3	1407.405	2	NC	1
501	4	max	0	9	.067	3	.017	3	7.093e-3	2	NC	4	NC	1
502		min	0	3	-.107	2	-.01	2	-5.277e-3	3	1055.103	2	NC	1
503	5	max	0	9	.074	3	.019	3	8.06e-3	2	NC	4	NC	1
504		min	0	3	-.118	2	-.012	2	-5.969e-3	3	930.15	2	7671.455	3
505	6	max	0	9	.076	3	.022	3	9.027e-3	2	NC	4	NC	1
506		min	0	3	-.119	2	-.015	2	-6.661e-3	3	915.825	2	6343.288	3
507	7	max	0	9	.074	3	.023	3	9.994e-3	2	NC	4	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.113	2	-.019	2	-7.353e-3	3	984.319	2	5614.877	3
509	8	max	0	9	.07	3	.024	3	1.096e-2	2	NC	4	NC	1
510		min	0	3	-.102	2	-.023	2	-8.045e-3	3	1132.311	2	5261.858	3
511	9	max	0	9	.065	3	.025	3	1.193e-2	2	NC	4	NC	4
512		min	0	3	-.09	2	-.026	2	-8.737e-3	3	1337.165	2	4959.663	2
513	10	max	0	9	.062	3	.024	3	1.289e-2	2	NC	4	NC	4
514		min	0	3	-.085	2	-.027	2	-9.429e-3	3	1464.98	2	4614.337	2
515	11	max	0	9	.065	3	.022	3	1.193e-2	2	NC	4	NC	4
516		min	0	3	-.09	2	-.026	2	-8.734e-3	3	1337.165	2	4959.669	2
517	12	max	0	9	.07	3	.021	3	1.096e-2	2	NC	4	NC	1
518		min	0	3	-.102	2	-.023	2	-8.039e-3	3	1132.311	2	6023.398	2
519	13	max	0	9	.074	3	.019	3	9.994e-3	2	NC	4	NC	1
520		min	0	3	-.113	2	-.019	2	-7.344e-3	3	984.319	2	7788.193	3
521	14	max	0	9	.076	3	.017	3	9.027e-3	2	NC	4	NC	1
522		min	0	3	-.119	2	-.015	2	-6.649e-3	3	915.825	2	9545.872	3
523	15	max	0	9	.074	3	.015	3	8.061e-3	2	NC	4	NC	1
524		min	0	3	-.118	2	-.012	2	-5.954e-3	3	930.15	2	NC	1
525	16	max	0	9	.067	3	.013	3	7.094e-3	2	NC	4	NC	1
526		min	0	3	-.107	2	-.01	2	-5.259e-3	3	1055.103	2	NC	1
527	17	max	0	9	.055	3	.011	3	6.127e-3	2	NC	4	NC	1
528		min	0	3	-.087	2	-.009	2	-4.564e-3	3	1407.405	2	NC	1
529	18	max	0	9	.039	3	.009	3	5.16e-3	2	NC	4	NC	1
530		min	0	3	-.06	2	-.009	2	-3.869e-3	3	2616.303	2	NC	1
531	19	max	0	9	.021	3	.008	3	4.193e-3	2	NC	1	NC	1
532		min	0	3	-.028	2	-.009	2	-3.175e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	4.08e-4	3	NC	1	NC	1
534		min	0	1	0	0	0	1	-4.469e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.83e-4	3	NC	1	NC	1
536		min	0	2	-.002	4	0	3	-4.08e-4	2	NC	1	NC	1
537	3	max	0	3	0	15	.002	2	1.158e-3	3	NC	1	NC	1
538		min	0	2	-.004	4	-.003	3	-7.713e-4	2	NC	1	8853.526	3
539	4	max	0	3	-.001	15	.005	2	1.533e-3	3	NC	1	NC	4
540		min	0	2	-.006	4	-.007	3	-1.135e-3	2	NC	1	4899.738	3
541	5	max	0	3	-.002	15	.009	2	1.908e-3	3	NC	1	NC	4
542		min	0	2	-.007	4	-.011	3	-1.498e-3	2	8038.769	4	3222.664	3
543	6	max	0	3	-.002	15	.013	2	2.283e-3	3	NC	3	NC	4
544		min	-.001	2	-.009	4	-.016	3	-1.861e-3	2	6765.474	4	2349.932	3
545	7	max	0	3	-.002	15	.017	2	2.658e-3	3	NC	3	NC	4
546		min	-.001	2	-.01	4	-.021	3	-2.224e-3	2	5999.755	4	1838.965	3
547	8	max	0	3	-.002	15	.02	2	3.032e-3	3	NC	5	NC	4
548		min	-.002	2	-.011	4	-.026	3	-2.588e-3	2	5540.208	4	1517.479	3
549	9	max	0	3	-.003	15	.024	2	3.407e-3	3	NC	5	NC	4
550		min	-.002	2	-.011	4	-.031	3	-2.951e-3	2	5292.853	4	1306.984	3
551	10	max	.001	3	-.003	15	.026	2	3.782e-3	3	NC	5	NC	4
552		min	-.002	2	-.011	4	-.034	3	-3.314e-3	2	5214.602	4	1168.023	3
553	11	max	.001	3	-.003	15	.028	2	4.157e-3	3	NC	5	NC	4
554		min	-.002	2	-.011	4	-.037	3	-3.678e-3	2	5292.853	4	1079.872	3
555	12	max	.001	3	-.002	15	.028	2	4.532e-3	3	NC	5	NC	4
556		min	-.002	2	-.011	4	-.037	3	-4.041e-3	2	5540.208	4	1032.258	3
557	13	max	.001	3	-.002	15	.027	2	4.907e-3	3	NC	3	NC	4
558		min	-.003	2	-.01	4	-.036	3	-4.404e-3	2	5999.755	4	1022.227	3
559	14	max	.001	3	-.001	2	.024	2	5.282e-3	3	NC	3	NC	4
560		min	-.003	2	-.009	4	-.033	3	-4.768e-3	2	6765.474	4	1054.274	3
561	15	max	.002	3	0	2	.019	2	5.657e-3	3	NC	1	NC	4
562		min	-.003	2	-.007	4	-.027	3	-5.131e-3	2	8038.769	4	1144.81	3
563	16	max	.002	3	.002	2	.013	1	6.032e-3	3	NC	1	NC	4
564		min	-.003	2	-.006	4	-.018	3	-5.494e-3	2	NC	1	1338.36	3



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.002	3	.004	2	.005	1	6.407e-3	3	NC	1	NC	4
566			min	-.004	2	-.004	4	-.006	3	-5.857e-3	2	NC	1	1774.581	3
567		18	max	.002	3	.006	2	.01	3	6.782e-3	3	NC	1	NC	4
568			min	-.004	2	-.002	4	-.012	2	-6.221e-3	2	NC	1	3159.921	3
569		19	max	.002	3	.008	2	.03	3	7.156e-3	3	NC	1	NC	1
570			min	-.004	2	0	9	-.028	2	-6.584e-3	2	NC	1	NC	1
571	M16A	1	max	.001	2	.002	2	.009	3	2.034e-3	3	NC	1	NC	1
572			min	-.002	3	-.002	3	-.009	2	-2.132e-3	2	NC	1	NC	1
573		2	max	.001	2	0	2	.002	3	1.96e-3	3	NC	1	NC	1
574			min	-.002	3	-.003	3	-.004	2	-2.034e-3	2	NC	1	8725.343	3
575		3	max	.001	2	0	2	.002	1	1.886e-3	3	NC	1	NC	4
576			min	-.002	3	-.004	4	-.003	3	-1.937e-3	2	NC	1	4941.598	3
577		4	max	.001	2	-.001	15	.005	1	1.812e-3	3	NC	1	NC	4
578			min	-.002	3	-.006	4	-.007	3	-1.839e-3	2	NC	1	3762.625	3
579		5	max	0	2	-.002	15	.007	1	1.738e-3	3	NC	1	NC	4
580			min	-.002	3	-.007	4	-.01	3	-1.742e-3	2	8038.769	4	3253.722	3
581		6	max	0	2	-.002	15	.008	1	1.664e-3	3	NC	3	NC	4
582			min	-.001	3	-.009	4	-.012	3	-1.644e-3	2	6765.474	4	3034.203	3
583		7	max	0	2	-.002	15	.008	1	1.59e-3	3	NC	3	NC	4
584			min	-.001	3	-.01	4	-.013	3	-1.547e-3	2	5999.755	4	2985.204	3
585		8	max	0	2	-.002	15	.008	1	1.516e-3	3	NC	5	NC	4
586			min	-.001	3	-.011	4	-.013	3	-1.449e-3	2	5540.208	4	3066.769	3
587		9	max	0	2	-.003	15	.008	1	1.442e-3	3	NC	5	NC	4
588			min	-.001	3	-.011	4	-.012	3	-1.352e-3	2	5292.853	4	3274.882	3
589		10	max	0	2	-.003	15	.007	1	1.368e-3	3	NC	5	NC	4
590			min	0	3	-.011	4	-.011	3	-1.254e-3	2	5214.602	4	3632.033	3
591		11	max	0	2	-.003	15	.006	1	1.294e-3	3	NC	5	NC	4
592			min	0	3	-.011	4	-.01	3	-1.156e-3	2	5292.853	4	4192.82	3
593		12	max	0	2	-.002	15	.005	1	1.22e-3	3	NC	5	NC	4
594			min	0	3	-.011	4	-.008	3	-1.059e-3	2	5540.208	4	5066.195	3
595		13	max	0	2	-.002	15	.004	1	1.146e-3	3	NC	3	NC	1
596			min	0	3	-.01	4	-.006	3	-9.614e-4	2	5999.755	4	6473.066	3
597		14	max	0	2	-.002	15	.003	1	1.072e-3	3	NC	3	NC	1
598			min	0	3	-.009	4	-.004	3	-8.638e-4	2	6765.474	4	8904.058	3
599		15	max	0	2	-.002	15	.001	1	9.982e-4	3	NC	1	NC	1
600			min	0	3	-.007	4	-.002	3	-7.663e-4	2	8038.769	4	NC	1
601		16	max	0	2	-.001	15	.001	4	9.242e-4	3	NC	1	NC	1
602			min	0	3	-.006	4	0	3	-6.687e-4	2	NC	1	NC	1
603		17	max	0	2	0	15	0	4	8.503e-4	3	NC	1	NC	1
604			min	0	3	-.004	4	0	2	-5.712e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	3	7.763e-4	3	NC	1	NC	1
606			min	0	3	-.002	4	0	2	-4.736e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	7.023e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-3.761e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

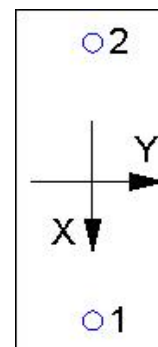
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.