

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	100 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 15.70 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad \text{(ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad \text{(ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

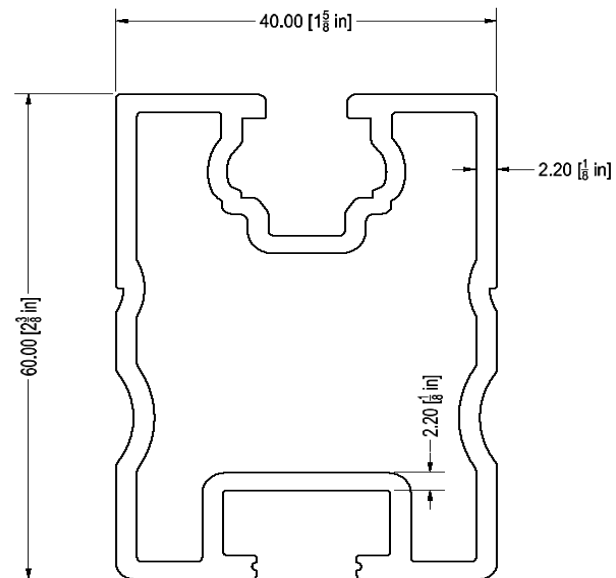
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

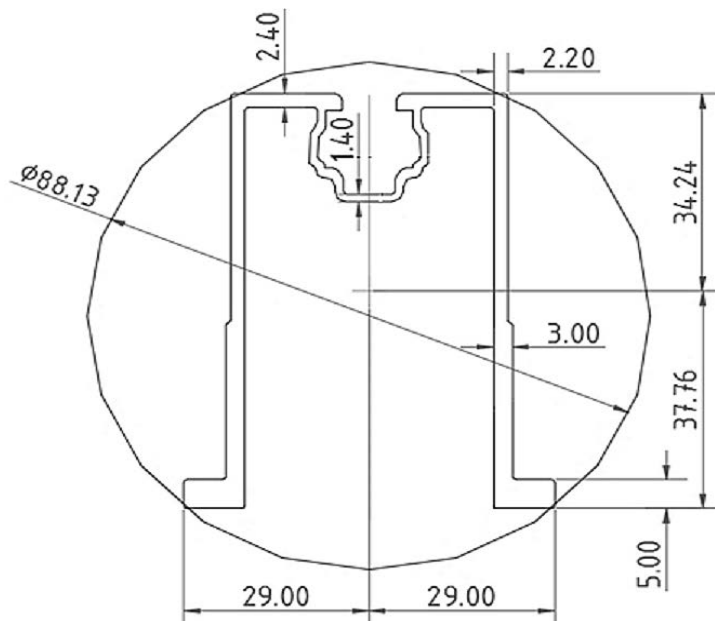
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	75 in
ΦF_{ty} STRONG-AXIS =	28.81 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.739 k-ft
M_z =	0.130 k-ft
$M_{y \text{ allowable}}$ =	1.226 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	75%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.46 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.550 k-ft
M_z =	0.000 k-ft
P_n =	0.280 k
$M_{y \text{ allowable}}$ =	1.446 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	40%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.043 k-ft
P_n =	0.237 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	12%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.334 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.026 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.005 k-ft
P_n =	0.213 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	13%



A cross brace kit is required every 15 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

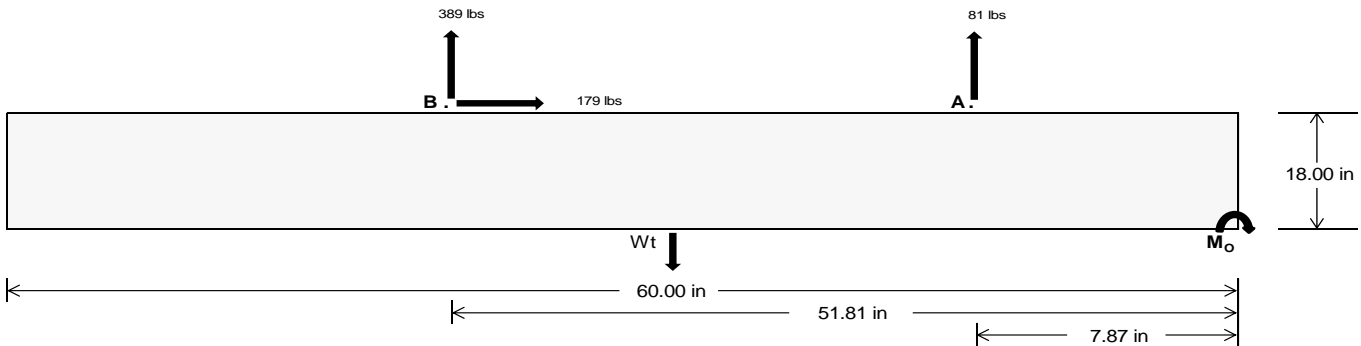
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	342.84	1621.75	k
Compressive Load =	1667.55	1301.07	k
Lateral Load =	34.91	745.16	k
Moment (Weak Axis) =	0.06	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24024.7$ in-lbs
Resisting Force Required = 800.82 lbs
S.F. = 1.67
Weight Required = 1334.71 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 179.08 lbs
Friction = 0.4
Weight Required = 447.70 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 179.08 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	598 lbs	598 lbs	598 lbs	598 lbs	543 lbs	543 lbs	543 lbs	543 lbs	811 lbs	811 lbs	811 lbs	811 lbs	-163 lbs	-163 lbs	-163 lbs	-163 lbs
F_B	436 lbs	436 lbs	436 lbs	436 lbs	478 lbs	478 lbs	478 lbs	478 lbs	653 lbs	653 lbs	653 lbs	653 lbs	-778 lbs	-778 lbs	-778 lbs	-778 lbs
F_V	52 lbs	52 lbs	52 lbs	52 lbs	320 lbs	320 lbs	320 lbs	320 lbs	275 lbs	275 lbs	275 lbs	275 lbs	-358 lbs	-358 lbs	-358 lbs	-358 lbs
P_{total}	3028 lbs	3118 lbs	3209 lbs	3300 lbs	3015 lbs	3105 lbs	3196 lbs	3287 lbs	3457 lbs	3547 lbs	3638 lbs	3729 lbs	255 lbs	310 lbs	364 lbs	418 lbs
M	386 lbs-ft	386 lbs-ft	386 lbs-ft	386 lbs-ft	610 lbs-ft	610 lbs-ft	610 lbs-ft	610 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	577 lbs-ft	577 lbs-ft	577 lbs-ft	577 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.26 ft	1.86 ft	1.59 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	279.7 psf	277.0 psf	274.5 psf	272.3 psf	249.0 psf	247.6 psf	246.3 psf	245.2 psf	282.7 psf	279.9 psf	277.3 psf	274.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	380.9 psf	373.8 psf	367.3 psf	361.3 psf	408.8 psf	400.5 psf	392.9 psf	385.8 psf	471.5 psf	460.5 psf	450.3 psf	441.0 psf	388.9 psf	169.5 psf	132.8 psf	119.5 psf

Maximum Bearing Pressure = 472 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

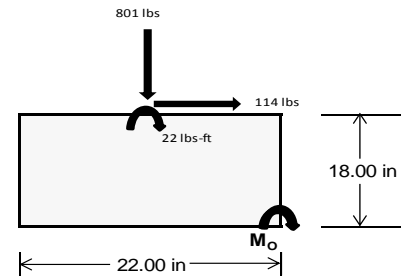
Overturning Check

$M_o = 540.9 \text{ ft-lbs}$
 Resisting Force Required = 590.09 lbs
 S.F. = 1.67
 Weight Required = 983.49 lbs
 Minimum Width = 22 in in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	124 lbs	138 lbs	72 lbs	318 lbs	801 lbs	277 lbs	73 lbs	1 lbs	23 lbs
F_v	19 lbs	151 lbs	19 lbs	13 lbs	114 lbs	15 lbs	19 lbs	151 lbs	19 lbs
P_{total}	2593 lbs	2606 lbs	2540 lbs	2668 lbs	3150 lbs	2627 lbs	795 lbs	723 lbs	745 lbs
M	54 lbs-ft	255 lbs-ft	57 lbs-ft	36 lbs-ft	193 lbs-ft	44 lbs-ft	54 lbs-ft	255 lbs-ft	57 lbs-ft
e	0.02 ft	0.10 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.07 ft	0.35 ft	0.08 ft
$L/6$	0.31 ft	1.64 ft	1.79 ft	1.81 ft	1.71 ft	1.80 ft	1.70 ft	1.13 ft	1.68 ft
f_{min}	263.5 sqft	193.2 sqft	256.8 sqft	278.1 sqft	274.8 sqft	270.8 sqft	67.3 sqft	-12.2 sqft	61.0 sqft
f_{max}	302.2 psf	375.4 psf	297.4 psf	304.0 psf	412.5 psf	302.3 psf	106.2 psf	169.9 psf	101.5 psf



Maximum Bearing Pressure = 413 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

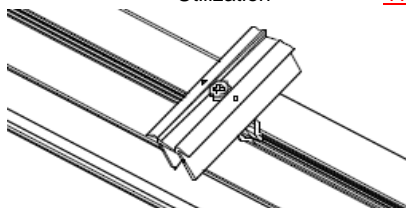
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

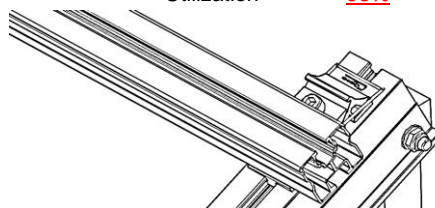
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.497 k
Allowable Uplift =	1.214 k
Utilization =	<u>41%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

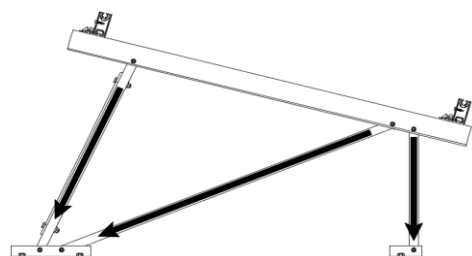
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.283 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

Diagonal Strut

Maximum Axial Load =	0.334 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.172 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.213 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.09 in
	<u>0.09 ≤ 0.591, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.255$$

$$195.296$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.255$$

$$202.803$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.7$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.226 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.13 \\ &23.1371 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.5 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.13 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b [Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.5 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b [Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.446 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} F_{cy}}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

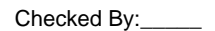
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 20° 100mph 30psf 6.25ft 7-05-2011 Page 20



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	316.978	1	.075	2	.506	1	0	10	.001	1	0	15
30			min	-358.431	3	.014	15	-.371	5	0	4	0	3	0	6
31		16	max	317.084	1	.043	2	.506	1	0	10	.001	1	0	15
32			min	-358.351	3	-.005	3	-.468	5	0	4	0	3	0	6
33		17	max	317.191	1	.011	2	.506	1	0	10	.001	1	0	15
34			min	-358.271	3	-.029	3	-.564	5	0	4	0	3	0	6
35		18	max	317.298	1	-.015	15	.506	1	0	10	.001	1	0	15
36			min	-358.191	3	-.06	4	-.661	5	0	4	0	3	0	6
37		19	max	317.404	1	-.025	15	.506	1	0	10	.001	1	0	15
38			min	-358.111	3	-.101	4	-.757	5	0	4	0	3	0	6
39	M3	1	max	75.684	2	1.794	6	-.028	12	0	5	.001	1	0	6
40			min	-85.011	3	.421	15	-1.406	4	0	1	0	12	0	15
41		2	max	75.616	2	1.616	6	-.028	12	0	5	.001	1	0	6
42			min	-85.062	3	.379	15	-1.272	4	0	1	0	12	0	15
43		3	max	75.549	2	1.439	6	-.028	12	0	5	.001	1	0	2
44			min	-85.112	3	.337	15	-1.139	4	0	1	0	15	0	3
45		4	max	75.481	2	1.261	6	-.028	12	0	5	.001	1	0	15
46			min	-85.163	3	.296	15	-1.005	4	0	1	0	5	0	4
47		5	max	75.413	2	1.083	6	-.028	12	0	5	0	1	0	15
48			min	-85.214	3	.254	15	-.871	4	0	1	0	5	0	4
49		6	max	75.345	2	.906	6	-.028	12	0	5	0	1	0	15
50			min	-85.265	3	.212	15	-.738	4	0	1	0	5	0	4
51		7	max	75.277	2	.728	6	-.028	12	0	5	0	1	0	15
52			min	-85.316	3	.17	15	-.604	4	0	1	0	5	0	4
53		8	max	75.209	2	.55	6	-.028	12	0	5	0	1	0	15
54			min	-85.367	3	.129	15	-.471	4	0	1	0	5	-.001	4
55		9	max	75.141	2	.373	6	-.028	12	0	5	0	1	0	15
56			min	-85.418	3	.087	15	-.436	1	0	1	0	5	-.001	4
57		10	max	75.074	2	.195	6	-.028	12	0	5	0	1	0	15
58			min	-85.469	3	.045	15	-.436	1	0	1	0	5	-.001	4
59		11	max	75.006	2	.032	2	.019	5	0	5	0	1	0	15
60			min	-85.52	3	-.003	3	-.436	1	0	1	0	5	-.001	4
61		12	max	74.938	2	-.038	15	.152	5	0	5	0	1	0	15
62			min	-85.571	3	-.16	4	-.436	1	0	1	0	5	-.001	4
63		13	max	74.87	2	-.08	15	.286	5	0	5	0	1	0	15
64			min	-85.621	3	-.338	4	-.436	1	0	1	0	5	-.001	4
65		14	max	74.802	2	-.122	15	.42	5	0	5	0	1	0	15
66			min	-85.672	3	-.516	4	-.436	1	0	1	0	5	-.001	4
67		15	max	74.734	2	-.164	15	.553	5	0	5	0	1	0	15
68			min	-85.723	3	-.693	4	-.436	1	0	1	0	5	0	4
69		16	max	74.666	2	-.205	15	.687	5	0	5	0	12	0	15
70			min	-85.774	3	-.871	4	-.436	1	0	1	0	4	0	4
71		17	max	74.598	2	-.247	15	.821	5	0	5	0	12	0	15
72			min	-85.825	3	-1.049	4	-.436	1	0	1	0	4	0	4
73		18	max	74.531	2	-.289	15	.954	5	0	5	0	12	0	15
74			min	-85.876	3	-1.226	4	-.436	1	0	1	0	1	0	4
75		19	max	74.463	2	-.331	15	1.088	5	0	5	0	5	0	1
76			min	-85.927	3	-1.404	4	-.436	1	0	1	0	1	0	1
77	M4	1	max	443.297	1	0	1	-.107	10	0	1	0	5	0	1
78			min	-73.681	3	0	1	-25.896	4	0	1	0	1	0	1
79		2	max	443.362	1	0	1	-.107	10	0	1	0	12	0	1
80			min	-73.633	3	0	1	-25.953	4	0	1	-.002	4	0	1
81		3	max	443.427	1	0	1	-.107	10	0	1	0	12	0	1
82			min	-73.584	3	0	1	-26.009	4	0	1	-.005	4	0	1
83		4	max	443.491	1	0	1	-.107	10	0	1	0	12	0	1
84			min	-73.536	3	0	1	-26.065	4	0	1	-.007	4	0	1
85		5	max	443.556	1	0	1	-.107	10	0	1	0	12	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-73.487	3	0	1	-26.121	4	0	1	-.009	4	0	1
87		6	max	443.621	1	0	1	-.107	10	0	1	0	12	0	1
88			min	-73.439	3	0	1	-26.177	4	0	1	-.012	4	0	1
89		7	max	443.685	1	0	1	-.107	10	0	1	0	12	0	1
90			min	-73.39	3	0	1	-26.233	4	0	1	-.014	4	0	1
91		8	max	443.75	1	0	1	-.107	10	0	1	0	12	0	1
92			min	-73.342	3	0	1	-26.289	4	0	1	-.016	4	0	1
93		9	max	443.815	1	0	1	-.107	10	0	1	0	12	0	1
94			min	-73.293	3	0	1	-26.345	4	0	1	-.019	4	0	1
95		10	max	443.88	1	0	1	-.107	10	0	1	0	12	0	1
96			min	-73.245	3	0	1	-26.401	4	0	1	-.021	4	0	1
97		11	max	443.944	1	0	1	-.107	10	0	1	0	12	0	1
98			min	-73.196	3	0	1	-26.457	4	0	1	-.023	4	0	1
99		12	max	444.009	1	0	1	-.107	10	0	1	0	10	0	1
100			min	-73.148	3	0	1	-26.513	4	0	1	-.026	4	0	1
101		13	max	444.074	1	0	1	-.107	10	0	1	0	10	0	1
102			min	-73.099	3	0	1	-26.569	4	0	1	-.028	4	0	1
103		14	max	444.138	1	0	1	-.107	10	0	1	0	10	0	1
104			min	-73.051	3	0	1	-26.625	4	0	1	-.031	4	0	1
105		15	max	444.203	1	0	1	-.107	10	0	1	0	10	0	1
106			min	-73.002	3	0	1	-26.682	4	0	1	-.033	4	0	1
107		16	max	444.268	1	0	1	-.107	10	0	1	0	10	0	1
108			min	-72.953	3	0	1	-26.738	4	0	1	-.035	4	0	1
109		17	max	444.333	1	0	1	-.107	10	0	1	0	10	0	1
110			min	-72.905	3	0	1	-26.794	4	0	1	-.038	4	0	1
111		18	max	444.397	1	0	1	-.107	10	0	1	0	10	0	1
112			min	-72.856	3	0	1	-26.85	4	0	1	-.04	4	0	1
113		19	max	444.462	1	0	1	-.107	10	0	1	0	10	0	1
114			min	-72.808	3	0	1	-26.906	4	0	1	-.042	4	0	1
115	M6	1	max	1023.778	1	.63	6	1.04	4	0	1	0	3	0	1
116			min	-1172.304	3	.144	15	-.169	3	0	5	0	1	0	1
117		2	max	1023.885	1	.589	6	.944	4	0	1	0	4	0	15
118			min	-1172.224	3	.135	15	-.169	3	0	5	0	1	0	6
119		3	max	1023.991	1	.547	6	.847	4	0	1	0	4	0	15
120			min	-1172.144	3	.125	15	-.169	3	0	5	0	1	0	6
121		4	max	1024.098	1	.506	6	.751	4	0	1	0	4	0	15
122			min	-1172.064	3	.115	15	-.169	3	0	5	0	10	0	6
123		5	max	1024.204	1	.465	6	.654	4	0	1	0	4	0	15
124			min	-1171.984	3	.105	15	-.169	3	0	5	0	3	0	6
125		6	max	1024.311	1	.424	6	.558	4	0	1	0	4	0	15
126			min	-1171.905	3	.096	15	-.169	3	0	5	0	3	0	6
127		7	max	1024.417	1	.383	2	.461	4	0	1	0	4	0	15
128			min	-1171.825	3	.086	15	-.169	3	0	5	0	3	0	6
129		8	max	1024.524	1	.351	2	.365	4	0	1	0	4	0	15
130			min	-1171.745	3	.076	15	-.169	3	0	5	0	3	0	6
131		9	max	1024.63	1	.319	2	.268	4	0	1	0	4	0	15
132			min	-1171.665	3	.067	15	-.169	3	0	5	0	3	0	6
133		10	max	1024.737	1	.287	2	.236	1	0	1	0	4	0	15
134			min	-1171.585	3	.057	15	-.169	3	0	5	0	3	0	6
135		11	max	1024.843	1	.255	2	.236	1	0	1	0	4	0	15
136			min	-1171.505	3	.047	15	-.169	3	0	5	0	3	0	6
137		12	max	1024.95	1	.222	2	.236	1	0	1	0	4	0	15
138			min	-1171.425	3	.035	12	-.169	3	0	5	0	3	0	6
139		13	max	1025.057	1	.19	2	.236	1	0	1	0	4	0	15
140			min	-1171.345	3	.019	12	-.17	5	0	5	0	3	0	2
141		14	max	1025.163	1	.158	2	.236	1	0	1	0	4	0	15
142			min	-1171.265	3	0	3	-.267	5	0	5	0	3	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1025.27	1	.126	2	.236	1	0	1	0	4	0	15
144		min	-1171.185	3	-.023	3	-.363	5	0	5	0	3	0	2
145	16	max	1025.376	1	.094	2	.236	1	0	1	0	4	0	15
146		min	-1171.105	3	-.047	3	-.46	5	0	5	0	3	0	2
147	17	max	1025.483	1	.062	2	.236	1	0	1	0	4	0	15
148		min	-1171.026	3	-.072	3	-.556	5	0	5	0	3	0	2
149	18	max	1025.589	1	.029	2	.236	1	0	1	0	4	0	15
150		min	-1170.946	3	-.096	3	-.653	5	0	5	0	3	0	2
151	19	max	1025.696	1	-.003	2	.236	1	0	1	0	1	0	15
152		min	-1170.866	3	-.12	3	-.749	5	0	5	0	3	0	2
153	M7	1	max	333.603	2	1.803	.007	1	0	1	0	4	0	2
154		min	-254.434	3	.428	15	-1.403	5	0	3	0	3	0	12
155	2	max	333.535	2	1.625	4	.007	1	0	1	0	4	0	2
156		min	-254.484	3	.386	15	-1.27	5	0	3	0	3	0	12
157	3	max	333.468	2	1.447	4	.007	1	0	1	0	4	0	2
158		min	-254.535	3	.344	15	-1.136	5	0	3	0	3	0	3
159	4	max	333.4	2	1.27	4	.007	1	0	1	0	1	0	2
160		min	-254.586	3	.302	15	-1.002	5	0	3	0	3	0	3
161	5	max	333.332	2	1.092	4	.007	1	0	1	0	1	0	15
162		min	-254.637	3	.261	15	-.869	5	0	3	0	5	0	6
163	6	max	333.264	2	.914	4	.007	1	0	1	0	1	0	15
164		min	-254.688	3	.219	15	-.735	5	0	3	0	5	0	6
165	7	max	333.196	2	.737	4	.007	1	0	1	0	1	0	15
166		min	-254.739	3	.177	15	-.602	5	0	3	0	5	0	6
167	8	max	333.128	2	.559	4	.007	1	0	1	0	1	0	15
168		min	-254.79	3	.135	15	-.468	5	0	3	0	5	0	6
169	9	max	333.06	2	.382	4	.007	1	0	1	0	1	0	15
170		min	-254.841	3	.094	15	-.334	5	0	3	0	5	-.001	6
171	10	max	332.993	2	.217	2	.007	1	0	1	0	1	0	15
172		min	-254.892	3	.043	12	-.201	5	0	3	0	5	-.001	6
173	11	max	332.925	2	.078	2	.007	1	0	1	0	1	0	15
174		min	-254.943	3	-.042	3	-.067	5	0	3	0	5	-.001	6
175	12	max	332.857	2	-.032	15	.067	4	0	1	0	1	0	15
176		min	-254.993	3	-.152	6	-.007	2	0	3	0	5	-.001	6
177	13	max	332.789	2	-.073	15	.201	4	0	1	0	1	0	15
178		min	-255.044	3	-.329	6	-.007	2	0	3	0	5	-.001	6
179	14	max	332.721	2	-.115	15	.335	4	0	1	0	1	0	15
180		min	-255.095	3	-.507	6	-.007	2	0	3	0	5	-.001	6
181	15	max	332.653	2	-.157	15	.468	4	0	1	0	1	0	15
182		min	-255.146	3	-.685	6	-.007	2	0	3	0	5	0	6
183	16	max	332.585	2	-.199	15	.602	4	0	1	0	1	0	15
184		min	-255.197	3	-.862	6	-.007	2	0	3	0	5	0	6
185	17	max	332.517	2	-.241	15	.736	4	0	1	0	1	0	15
186		min	-255.248	3	-1.04	6	-.007	2	0	3	0	5	0	6
187	18	max	332.45	2	-.282	15	.869	4	0	1	0	1	0	15
188		min	-255.299	3	-1.218	6	-.007	2	0	3	0	5	0	6
189	19	max	332.382	2	-.324	15	1.003	4	0	1	0	1	0	1
190		min	-255.35	3	-1.395	6	-.007	2	0	3	0	3	0	1
191	M8	1	max	1281.563	1	0	.613	1	0	1	0	4	0	1
192		min	-264.596	3	0	1	-26.186	4	0	1	0	1	0	1
193	2	max	1281.628	1	0	1	.613	1	0	1	0	1	0	1
194		min	-264.548	3	0	1	-26.242	4	0	1	-.002	4	0	1
195	3	max	1281.693	1	0	1	.613	1	0	1	0	1	0	1
196		min	-264.499	3	0	1	-26.298	4	0	1	-.005	4	0	1
197	4	max	1281.757	1	0	1	.613	1	0	1	0	1	0	1
198		min	-264.451	3	0	1	-26.354	4	0	1	-.007	4	0	1
199	5	max	1281.822	1	0	1	.613	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-264.402	3	0	1	-26.41	4	0	1	-.009	4	0	1
201		6	max	1281.887	1	0	1	.613	1	0	1	0	1	0	1
202			min	-264.354	3	0	1	-26.466	4	0	1	-.012	4	0	1
203		7	max	1281.952	1	0	1	.613	1	0	1	0	1	0	1
204			min	-264.305	3	0	1	-26.522	4	0	1	-.014	4	0	1
205		8	max	1282.016	1	0	1	.613	1	0	1	0	1	0	1
206			min	-264.257	3	0	1	-26.578	4	0	1	-.016	4	0	1
207		9	max	1282.081	1	0	1	.613	1	0	1	0	1	0	1
208			min	-264.208	3	0	1	-26.634	4	0	1	-.019	4	0	1
209		10	max	1282.146	1	0	1	.613	1	0	1	0	1	0	1
210			min	-264.16	3	0	1	-26.69	4	0	1	-.021	4	0	1
211		11	max	1282.21	1	0	1	.613	1	0	1	0	1	0	1
212			min	-264.111	3	0	1	-26.746	4	0	1	-.024	4	0	1
213		12	max	1282.275	1	0	1	.613	1	0	1	0	1	0	1
214			min	-264.063	3	0	1	-26.802	4	0	1	-.026	4	0	1
215		13	max	1282.34	1	0	1	.613	1	0	1	0	1	0	1
216			min	-264.014	3	0	1	-26.859	4	0	1	-.028	4	0	1
217		14	max	1282.404	1	0	1	.613	1	0	1	0	1	0	1
218			min	-263.965	3	0	1	-26.915	4	0	1	-.031	4	0	1
219		15	max	1282.469	1	0	1	.613	1	0	1	0	1	0	1
220			min	-263.917	3	0	1	-26.971	4	0	1	-.033	4	0	1
221		16	max	1282.534	1	0	1	.613	1	0	1	0	1	0	1
222			min	-263.868	3	0	1	-27.027	4	0	1	-.036	4	0	1
223		17	max	1282.599	1	0	1	.613	1	0	1	0	1	0	1
224			min	-263.82	3	0	1	-27.083	4	0	1	-.038	4	0	1
225		18	max	1282.663	1	0	1	.613	1	0	1	0	1	0	1
226			min	-263.771	3	0	1	-27.139	4	0	1	-.041	4	0	1
227		19	max	1282.728	1	0	1	.613	1	0	1	0	1	0	1
228			min	-263.723	3	0	1	-27.195	4	0	1	-.043	4	0	1
229	M10	1	max	319.514	1	.668	4	1.217	5	0	1	0	1	0	1
230			min	-340.203	3	.168	15	-.088	1	-.002	5	0	3	0	1
231		2	max	319.62	1	.627	4	1.12	5	0	1	0	4	0	15
232			min	-340.123	3	.158	15	-.088	1	-.002	5	0	3	0	4
233		3	max	319.727	1	.586	4	1.024	5	0	1	0	4	0	15
234			min	-340.043	3	.148	15	-.088	1	-.002	5	0	3	0	4
235		4	max	319.834	1	.544	4	.927	5	0	1	0	4	0	15
236			min	-339.964	3	.139	15	-.088	1	-.002	5	0	3	0	4
237		5	max	319.94	1	.503	4	.831	5	0	1	0	4	0	15
238			min	-339.884	3	.129	15	-.088	1	-.002	5	0	3	0	4
239		6	max	320.047	1	.462	4	.734	5	0	1	0	4	0	15
240			min	-339.804	3	.119	15	-.088	1	-.002	5	0	3	0	4
241		7	max	320.153	1	.421	4	.638	5	0	1	0	4	0	15
242			min	-339.724	3	.11	15	-.088	1	-.002	5	0	3	0	4
243		8	max	320.26	1	.379	4	.541	5	0	1	0	4	0	15
244			min	-339.644	3	.1	15	-.088	1	-.002	5	0	3	0	4
245		9	max	320.366	1	.338	4	.445	5	0	1	.001	4	0	15
246			min	-339.564	3	.09	15	-.088	1	-.002	5	0	3	0	4
247		10	max	320.473	1	.297	4	.349	5	0	1	.001	4	0	15
248			min	-339.484	3	.081	15	-.088	1	-.002	5	0	3	0	4
249		11	max	320.579	1	.256	4	.252	5	0	1	.001	4	0	15
250			min	-339.404	3	.071	15	-.088	1	-.002	5	0	3	0	4
251		12	max	320.686	1	.214	4	.156	5	0	1	.001	4	0	15
252			min	-339.324	3	.061	15	-.088	1	-.002	5	0	3	0	4
253		13	max	320.792	1	.173	4	.059	5	0	1	.001	4	0	15
254			min	-339.244	3	.051	15	-.088	1	-.002	5	0	3	0	4
255		14	max	320.899	1	.132	4	-.022	12	0	1	.001	4	0	15
256			min	-339.164	3	.029	1	-.088	1	-.002	5	0	3	0	4



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	321.006	1	.091	4	-.022	12	0	1	.001	4	0	15
258		min	-339.085	3	-.003	1	-.143	4	-.002	5	0	3	0	4
259	16	max	321.112	1	.049	4	-.022	12	0	1	.001	5	0	15
260		min	-339.005	3	-.035	1	-.239	4	-.002	5	0	3	0	4
261	17	max	321.219	1	.019	5	-.022	12	0	1	.001	5	0	15
262		min	-338.925	3	-.068	1	-.336	4	-.002	5	0	3	0	4
263	18	max	321.325	1	.004	5	-.022	12	0	1	.001	5	0	15
264		min	-338.845	3	-.1	1	-.432	4	-.002	5	0	3	0	4
265	19	max	321.432	1	-.007	15	-.022	12	0	1	0	5	0	15
266		min	-338.765	3	-.132	1	-.529	4	-.002	5	0	3	0	4
267	M11	1	max	75.343	2	1.79	.535	1	.001	4	.001	5	0	6
268		min	-85.614	3	.418	15	-1.184	5	0	10	-.001	1	0	15
269	2	max	75.276	2	1.613	6	.535	1	.001	4	0	5	0	6
270		min	-85.665	3	.377	15	-1.05	5	0	10	-.001	1	0	12
271	3	max	75.208	2	1.435	6	.535	1	.001	4	0	5	0	2
272		min	-85.716	3	.335	15	-.917	5	0	10	-.001	1	0	3
273	4	max	75.14	2	1.257	6	.535	1	.001	4	0	5	0	15
274		min	-85.767	3	.293	15	-.783	5	0	10	-.001	1	0	4
275	5	max	75.072	2	1.08	6	.535	1	.001	4	0	5	0	15
276		min	-85.817	3	.251	15	-.649	5	0	10	-.001	1	0	4
277	6	max	75.004	2	.902	6	.535	1	.001	4	0	5	0	15
278		min	-85.868	3	.21	15	-.516	5	0	10	0	1	0	4
279	7	max	74.936	2	.724	6	.535	1	.001	4	0	3	0	15
280		min	-85.919	3	.168	15	-.382	5	0	10	0	1	0	4
281	8	max	74.868	2	.547	6	.535	1	.001	4	0	3	0	15
282		min	-85.97	3	.126	15	-.248	5	0	10	0	1	-.001	4
283	9	max	74.801	2	.369	6	.535	1	.001	4	0	3	0	15
284		min	-86.021	3	.084	15	-.115	5	0	10	0	1	-.001	4
285	10	max	74.733	2	.191	6	.535	1	.001	4	0	3	0	15
286		min	-86.072	3	.043	15	0	3	0	10	0	1	-.001	4
287	11	max	74.665	2	.032	2	.535	1	.001	4	0	3	0	15
288		min	-86.123	3	-.022	3	0	3	0	10	0	1	-.001	4
289	12	max	74.597	2	-.041	15	.535	1	.001	4	0	3	0	15
290		min	-86.174	3	-.164	4	0	3	0	10	0	1	-.001	4
291	13	max	74.529	2	-.083	15	.535	1	.001	4	0	3	0	15
292		min	-86.225	3	-.342	4	0	3	0	10	0	1	-.001	4
293	14	max	74.461	2	-.124	15	.654	4	.001	4	0	4	0	15
294		min	-86.276	3	-.52	4	0	3	0	10	0	10	-.001	4
295	15	max	74.393	2	-.166	15	.788	4	.001	4	0	4	0	15
296		min	-86.326	3	-.697	4	0	3	0	10	0	10	0	4
297	16	max	74.326	2	-.208	15	.922	4	.001	4	0	4	0	15
298		min	-86.377	3	-.875	4	0	3	0	10	0	10	0	4
299	17	max	74.258	2	-.25	15	1.055	4	.001	4	0	4	0	15
300		min	-86.428	3	-1.052	4	0	3	0	10	0	10	0	4
301	18	max	74.19	2	-.292	15	1.189	4	.001	4	.001	4	0	15
302		min	-86.479	3	-1.23	4	0	3	0	10	0	10	0	4
303	19	max	74.122	2	-.333	15	1.322	4	.001	4	.001	4	0	1
304		min	-86.53	3	-1.408	4	0	3	0	10	0	10	0	1
305	M12	1	max	443.071	1	0	1	2.332	1	0	1	0	4	1
306		min	-73.272	3	0	1	-23.958	5	0	1	0	3	0	1
307	2	max	443.135	1	0	1	2.332	1	0	1	0	1	0	1
308		min	-73.223	3	0	1	-24.014	5	0	1	-.002	5	0	1
309	3	max	443.2	1	0	1	2.332	1	0	1	0	1	0	1
310		min	-73.175	3	0	1	-24.07	5	0	1	-.004	5	0	1
311	4	max	443.265	1	0	1	2.332	1	0	1	0	1	0	1
312		min	-73.126	3	0	1	-24.127	5	0	1	-.006	5	0	1
313	5	max	443.329	1	0	1	2.332	1	0	1	0	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	88.585	1	3.321	9	-2.329	12	0	5	-.002	12	.16	2
372			min	3.793	10	-22.342	3	-46.026	1	0	1	-.05	1	-.091	3
373		16	max	80.1	2	33.66	2	-2.358	12	0	1	-.003	12	.163	2
374			min	-30.916	3	-84.97	3	-46.412	1	0	5	-.06	1	-.085	3
375		17	max	80.195	2	33.398	2	-2.358	12	0	1	-.003	12	.157	1
376			min	-30.844	3	-85.167	3	-46.412	1	0	5	-.07	1	-.067	3
377		18	max	-3.71	12	365.672	1	-2.492	12	0	3	-.004	12	.08	1
378			min	-104.425	1	-154.669	3	-47.478	1	0	1	-.081	1	-.034	3
379		19	max	-3.662	12	365.409	1	-2.492	12	0	3	-.004	12	0	1
380			min	-104.33	1	-154.865	3	-47.478	1	0	1	-.091	1	0	3
381	M5	1	max	233.307	1	1119.431	3	0	10	0	1	.039	4	0	3
382			min	4.544	15	-1044.151	1	-36.724	3	0	5	0	10	0	1
383		2	max	233.402	1	1119.234	3	0	10	0	1	.033	4	.226	1
384			min	4.573	15	-1044.414	1	-36.724	3	0	5	-.004	3	-.242	3
385		3	max	176.832	1	7.388	9	4.191	3	0	3	.028	4	.448	1
386			min	1.752	10	-71.012	3	-21.065	4	0	4	-.011	3	-.48	3
387		4	max	176.928	1	7.17	9	4.191	3	0	3	.023	4	.453	1
388			min	1.831	10	-71.209	3	-20.823	4	0	4	-.01	3	-.465	3
389		5	max	177.023	1	6.951	9	4.191	3	0	3	.019	4	.458	1
390			min	1.911	10	-71.405	3	-20.581	4	0	4	-.009	3	-.449	3
391		6	max	177.119	1	6.732	9	4.191	3	0	3	.014	4	.463	1
392			min	1.991	10	-71.602	3	-20.339	4	0	4	-.009	3	-.434	3
393		7	max	177.214	1	6.514	9	4.191	3	0	3	.01	4	.468	1
394			min	2.07	10	-71.799	3	-20.097	4	0	4	-.008	3	-.418	3
395		8	max	177.31	1	6.295	9	4.191	3	0	3	.006	4	.473	1
396			min	2.15	10	-71.996	3	-19.855	4	0	4	-.007	3	-.402	3
397		9	max	177.405	1	6.076	9	4.191	3	0	3	.001	5	.479	1
398			min	2.229	10	-72.193	3	-19.613	4	0	4	-.006	3	-.387	3
399		10	max	177.501	1	5.858	9	4.191	3	0	3	0	10	.484	1
400			min	2.309	10	-72.389	3	-19.371	4	0	4	-.005	3	-.371	3
401		11	max	177.596	1	5.639	9	4.191	3	0	3	0	10	.489	1
402			min	2.389	10	-72.586	3	-19.129	4	0	4	-.007	4	-.355	3
403		12	max	177.692	1	5.42	9	4.191	3	0	3	0	10	.496	2
404			min	2.468	10	-72.783	3	-18.887	4	0	4	-.011	4	-.34	3
405		13	max	177.787	1	5.202	9	4.191	3	0	3	0	10	.51	2
406			min	2.548	10	-72.98	3	-18.645	4	0	4	-.015	4	-.324	3
407		14	max	177.883	1	4.983	9	4.191	3	0	3	0	10	.524	2
408			min	2.627	10	-73.177	3	-18.403	4	0	4	-.019	4	-.308	3
409		15	max	177.978	1	4.764	9	4.191	3	0	3	0	10	.538	2
410			min	2.707	10	-73.373	3	-18.161	4	0	4	-.023	4	-.292	3
411		16	max	280.147	2	176.603	2	4.16	3	0	1	0	3	.55	2
412			min	-100.302	3	-257.408	3	-16.924	4	0	4	-.027	4	-.275	3
413		17	max	280.243	2	176.341	2	4.16	3	0	1	.001	3	.516	1
414			min	-100.231	3	-257.605	3	-16.682	4	0	4	-.031	4	-.219	3
415		18	max	-6.666	12	1201.513	1	3.814	3	0	4	.002	3	.26	1
416			min	-233.544	1	-507.222	3	-41.807	5	0	1	-.04	4	-.11	3
417		19	max	-6.618	12	1201.251	1	3.814	3	0	4	.003	3	0	3
418			min	-233.448	1	-507.418	3	-41.565	5	0	1	-.049	4	0	1
419	M9	1	max	104.016	1	339.376	3	172.006	4	0	3	0	15	0	1
420			min	1.457	15	-316.566	1	3.321	10	0	1	-.09	1	0	3
421		2	max	104.111	1	339.18	3	172.248	4	0	3	.035	5	.069	1
422			min	1.485	15	-316.828	1	3.321	10	0	1	-.079	1	-.074	3
423		3	max	87.487	1	5.922	9	44.574	1	0	1	.068	5	.136	1
424			min	1.375	15	-19.916	3	-28.036	5	0	10	-.067	1	-.146	3
425		4	max	87.582	1	5.703	9	44.574	1	0	1	.062	5	.137	1
426			min	1.404	15	-20.112	3	-27.794	5	0	10	-.058	1	-.141	3
427		5	max	87.678	1	5.485	9	44.574	1	0	1	.056	5	.137	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	1.433	15	-20.309	3	-27.552	5	0	10	-.048	1	-.137	3
429	6	max	87.773	1	5.266	9	44.574	1	0	1	.05	5	.138	1
430		min	1.461	15	-20.506	3	-27.31	5	0	10	-.038	1	-.133	3
431	7	max	87.869	1	5.047	9	44.574	1	0	1	.044	5	.138	1
432		min	1.49	15	-20.703	3	-27.068	5	0	10	-.029	1	-.128	3
433	8	max	87.964	1	4.829	9	44.574	1	0	1	.038	5	.139	1
434		min	1.519	15	-20.9	3	-26.826	5	0	10	-.019	1	-.124	3
435	9	max	88.06	1	4.61	9	44.574	1	0	1	.033	5	.14	1
436		min	1.548	15	-21.096	3	-26.584	5	0	10	-.009	1	-.119	3
437	10	max	88.155	1	4.391	9	44.574	1	0	1	.027	4	.14	1
438		min	1.577	15	-21.293	3	-26.342	5	0	10	0	2	-.115	3
439	11	max	88.251	1	4.173	9	44.574	1	0	1	.023	4	.143	2
440		min	1.605	15	-21.49	3	-26.1	5	0	10	0	10	-.11	3
441	12	max	88.346	1	3.954	9	44.574	1	0	1	.02	1	.147	2
442		min	1.634	15	-21.687	3	-25.858	5	0	10	.001	10	-.105	3
443	13	max	88.442	1	3.735	9	44.574	1	0	1	.029	1	.151	2
444		min	1.663	15	-21.884	3	-25.616	5	0	10	.002	10	-.1	3
445	14	max	88.537	1	3.517	9	44.574	1	0	1	.039	1	.155	2
446		min	1.692	15	-22.08	3	-25.374	5	0	10	.003	10	-.096	3
447	15	max	88.633	1	3.298	9	44.574	1	0	1	.049	1	.159	2
448		min	1.721	15	-22.277	3	-25.132	5	0	10	-.001	5	-.091	3
449	16	max	80.287	2	33.404	2	45.07	1	0	10	.059	1	.163	2
450		min	-31.156	3	-85.352	3	-23.673	5	0	4	-.005	5	-.086	3
451	17	max	80.382	2	33.141	2	45.07	1	0	10	.069	1	.157	1
452		min	-31.085	3	-85.549	3	-23.431	5	0	4	-.01	5	-.067	3
453	18	max	4.394	5	365.673	1	47.377	1	0	2	.079	1	.08	1
454		min	-104.075	1	-154.666	3	-47.099	5	0	3	-.02	5	-.034	3
455	19	max	4.439	5	365.411	1	47.377	1	0	2	.09	1	0	1
456		min	-103.979	1	-154.862	3	-46.857	5	0	3	-.03	5	0	3
457	M13	1	max	172.01	4	316.203	1	-1.456	15	0	.09	1	0	1
458		min	3.321	10	-339.38	3	-104.006	1	0	3	0	15	0	3
459	2	max	165.189	4	223.232	1	-.645	15	0	1	.026	1	.201	3
460		min	3.321	10	-239.488	3	-79.464	1	0	3	-.001	5	-.187	1
461	3	max	158.369	4	130.262	1	.167	15	0	1	.003	3	.333	3
462		min	3.321	10	-139.595	3	-54.922	1	0	3	-.021	1	-.31	1
463	4	max	151.548	4	37.291	1	1.399	5	0	1	0	3	.395	3
464		min	3.321	10	-39.703	3	-30.38	1	0	3	-.05	1	-.368	1
465	5	max	144.727	4	60.19	3	2.655	5	0	1	0	5	.388	3
466		min	3.321	10	-55.679	1	-5.838	1	0	3	-.063	1	-.362	1
467	6	max	137.907	4	160.082	3	18.704	1	0	1	.003	5	.311	3
468		min	3.321	10	-148.65	1	-.401	3	0	3	-.058	1	-.291	1
469	7	max	131.086	4	259.975	3	43.246	1	0	1	.006	5	.165	3
470		min	3.321	10	-241.621	1	.573	12	0	3	-.037	1	-.155	1
471	8	max	124.266	4	359.867	3	67.789	1	0	1	.01	4	.045	1
472		min	3.321	10	-334.591	1	1.361	12	0	3	0	3	-.05	3
473	9	max	117.445	4	459.76	3	92.331	1	0	1	.057	1	.309	1
474		min	3.321	10	-427.562	1	2.149	12	0	3	.001	12	-.334	3
475	10	max	110.624	4	559.652	3	116.873	1	0	1	.13	1	.639	1
476		min	3.321	10	-520.532	1	2.936	12	0	3	.003	12	-.688	3
477	11	max	79.625	4	427.562	1	2.971	5	0	3	.056	1	.309	1
478		min	2.281	12	-459.76	3	-91.911	1	0	1	-.016	5	-.334	3
479	12	max	72.804	4	334.591	1	4.227	5	0	3	.002	2	.045	1
480		min	2.281	12	-359.867	3	-67.369	1	0	1	-.013	4	-.05	3
481	13	max	65.984	4	241.62	1	5.483	5	0	3	-.003	10	.165	3
482		min	2.281	12	-259.975	3	-42.827	1	0	1	-.037	1	-.155	1
483	14	max	59.163	4	148.65	1	6.739	5	0	3	-.003	12	.311	3
484		min	2.281	12	-160.082	3	-18.285	1	0	1	-.059	1	-.291	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	52.342	4	55.679	1	9.44	4	0	3	0	15	.388	3
486		min	2.281	12	-60.19	3	-.033	10	0	1	-.063	1	-.362	1
487	16	max	46.407	1	39.703	3	30.799	1	0	3	.006	5	.395	3
488		min	2.281	12	-37.291	1	1.968	12	0	1	-.05	1	-.368	1
489	17	max	46.407	1	139.595	3	55.342	1	0	3	.013	5	.333	3
490		min	2.281	12	-130.262	1	2.756	12	0	1	-.02	1	-.31	1
491	18	max	46.407	1	239.488	3	79.884	1	0	3	.027	1	.201	3
492		min	2.281	12	-223.233	1	3.543	12	0	1	.001	10	-.187	1
493	19	max	46.407	1	339.38	3	104.426	1	0	3	.091	1	0	1
494		min	2.281	12	-316.203	1	4.331	12	0	1	.005	12	0	3
495	M16	1	max	46.847	5	365.794	1	4.439	5	0	.09	1	0	2
496		min	-47.255	1	-154.879	3	-103.989	1	0	1	-.03	5	0	3
497	2	max	40.026	5	258.229	1	5.695	5	0	3	.026	1	.092	3
498		min	-47.255	1	-109.475	3	-79.447	1	0	1	-.027	5	-.217	1
499	3	max	33.205	5	150.663	1	6.951	5	0	3	0	12	.152	3
500		min	-47.255	1	-64.071	3	-54.905	1	0	1	-.026	4	-.359	1
501	4	max	26.385	5	43.098	1	8.207	5	0	3	-.002	12	.181	3
502		min	-47.255	1	-18.668	3	-30.363	1	0	1	-.05	1	-.426	1
503	5	max	19.564	5	26.736	3	9.463	5	0	3	-.003	12	.178	3
504		min	-47.255	1	-64.468	1	-5.82	1	0	1	-.063	1	-.418	1
505	6	max	12.744	5	72.139	3	18.722	1	0	3	-.002	15	.144	3
506		min	-47.255	1	-172.034	1	.005	3	0	1	-.059	1	-.336	1
507	7	max	5.923	5	117.543	3	43.264	1	0	3	.004	5	.078	3
508		min	-47.255	1	-279.599	1	.827	12	0	1	-.037	1	-.18	1
509	8	max	-.015	3	162.946	3	67.806	1	0	3	.013	4	.052	1
510		min	-47.255	1	-387.165	1	1.615	12	0	1	-.003	3	-.02	3
511	9	max	-.015	3	208.35	3	92.348	1	0	3	.057	1	.358	1
512		min	-47.255	1	-494.73	1	2.403	12	0	1	0	3	-.149	3
513	10	max	27.043	5	-12.486	15	116.89	1	0	14	.13	1	.739	1
514		min	-47.367	1	-602.296	1	-5.113	3	0	1	.003	12	-.309	3
515	11	max	20.223	5	494.73	1	2.832	5	0	1	.057	1	.358	1
516		min	-47.367	1	-208.35	3	-91.998	1	0	3	-.014	5	-.149	3
517	12	max	13.402	5	387.165	1	4.088	5	0	1	.002	2	.052	1
518		min	-47.367	1	-162.946	3	-67.456	1	0	3	-.011	4	-.02	3
519	13	max	6.582	5	279.599	1	5.344	5	0	1	-.001	12	.078	3
520		min	-47.367	1	-117.543	3	-42.913	1	0	3	-.037	1	-.18	1
521	14	max	-.096	15	172.033	1	6.6	5	0	1	-.002	12	.144	3
522		min	-47.367	1	-72.139	3	-18.371	1	0	3	-.058	1	-.336	1
523	15	max	-2.492	12	64.468	1	9.277	4	0	1	.001	5	.178	3
524		min	-47.367	1	-26.736	3	-.031	10	0	3	-.063	1	-.418	1
525	16	max	-2.492	12	18.668	3	30.713	1	0	1	.007	5	.181	3
526		min	-47.367	1	-43.098	1	1.299	12	0	3	-.05	1	-.426	1
527	17	max	-2.492	12	64.071	3	55.255	1	0	1	.014	5	.152	3
528		min	-47.367	1	-150.663	1	2.086	12	0	3	-.02	1	-.359	1
529	18	max	-2.492	12	109.475	3	79.797	1	0	1	.027	4	.092	3
530		min	-47.367	1	-258.229	1	2.874	12	0	3	.001	10	-.217	1
531	19	max	-2.492	12	154.879	3	104.339	1	0	1	.091	1	0	1
532		min	-47.367	1	-365.795	1	3.662	12	0	3	.004	12	0	3
533	M15	1	max	0	1.701	1	.051	3	0	1	0	1	0	1
534		min	-44.512	3	0	2	-.054	1	0	3	0	3	0	1
535	2	max	0	2	1.512	1	.051	3	0	1	0	1	0	2
536		min	-44.572	3	0	2	-.054	1	0	3	0	3	0	1
537	3	max	0	2	1.323	1	.051	3	0	1	0	1	0	2
538		min	-44.631	3	0	2	-.054	1	0	3	0	3	-.001	1
539	4	max	0	2	1.134	1	.051	3	0	1	0	1	0	2
540		min	-44.691	3	0	2	-.054	1	0	3	0	3	-.002	1
541	5	max	0	2	.945	1	.051	3	0	1	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-44.751	3	0	2	-.054	1	0	3	0	3	-.002	1
543		6	max	0	2	.756	1	.051	3	0	1	0	1	0	2
544			min	-44.81	3	0	2	-.054	1	0	3	0	3	-.002	1
545		7	max	0	2	.567	1	.051	3	0	1	0	3	0	2
546			min	-44.87	3	0	2	-.054	1	0	3	0	1	-.003	1
547		8	max	0	2	.378	1	.051	3	0	1	0	3	0	2
548			min	-44.93	3	0	2	-.054	1	0	3	0	1	-.003	1
549		9	max	0	2	.189	1	.051	3	0	1	0	3	0	2
550			min	-44.989	3	0	2	-.054	1	0	3	0	1	-.003	1
551		10	max	0	2	0	1	.051	3	0	1	0	3	0	2
552			min	-45.049	3	0	1	-.054	1	0	3	0	1	-.003	1
553		11	max	0	2	0	2	.051	3	0	1	0	3	0	2
554			min	-45.109	3	-.189	1	-.054	1	0	3	0	1	-.003	1
555		12	max	0	2	0	2	.051	3	0	1	0	3	0	2
556			min	-45.168	3	-.378	1	-.054	1	0	3	0	1	-.003	1
557		13	max	0	2	0	2	.051	3	0	1	0	3	0	2
558			min	-45.228	3	-.567	1	-.054	1	0	3	0	1	-.003	1
559		14	max	0	2	0	2	.051	3	0	1	0	3	0	2
560			min	-45.288	3	-.756	1	-.054	1	0	3	0	1	-.002	1
561		15	max	0	2	0	2	.051	3	0	1	0	3	0	2
562			min	-45.347	3	-.945	1	-.054	1	0	3	0	1	-.002	1
563		16	max	0	2	0	2	.051	3	0	1	0	3	0	2
564			min	-45.407	3	-1.134	1	-.054	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.051	3	0	1	0	3	0	2
566			min	-45.467	3	-1.323	1	-.054	1	0	3	0	1	-.001	1
567		18	max	0	2	0	2	.051	3	0	1	0	3	0	2
568			min	-45.526	3	-1.512	1	-.054	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.051	3	0	1	0	3	0	1
570			min	-45.586	3	-1.701	1	-.054	1	0	3	0	1	0	1
571	M16A	1	max	0	10	2.799	4	.237	4	0	3	0	3	0	1
572			min	-212.082	4	0	10	-.02	3	0	1	0	4	0	1
573		2	max	0	10	2.488	4	.215	4	0	3	0	3	0	10
574			min	-212.142	4	0	10	-.02	3	0	1	0	4	-.001	4
575		3	max	0	10	2.177	4	.192	4	0	3	0	3	0	10
576			min	-212.203	4	0	10	-.02	3	0	1	0	4	-.002	4
577		4	max	0	10	1.866	4	.169	4	0	3	0	3	0	10
578			min	-212.263	4	0	10	-.02	3	0	1	0	4	-.003	4
579		5	max	0	10	1.555	4	.147	4	0	3	0	3	0	10
580			min	-212.323	4	0	10	-.02	3	0	1	0	1	-.003	4
581		6	max	0	10	1.244	4	.124	4	0	3	0	3	0	10
582			min	-212.384	4	0	10	-.02	3	0	1	0	1	-.004	4
583		7	max	0	10	.933	4	.102	4	0	3	0	5	0	10
584			min	-212.444	4	0	10	-.02	3	0	1	0	1	-.004	4
585		8	max	0	10	.622	4	.079	4	0	3	0	5	0	10
586			min	-212.505	4	0	10	-.02	3	0	1	0	1	-.005	4
587		9	max	0	10	.311	4	.056	4	0	3	0	5	0	10
588			min	-212.565	4	0	10	-.02	3	0	1	0	1	-.005	4
589		10	max	0	10	0	1	.034	4	0	3	0	5	0	10
590			min	-212.625	4	0	1	-.02	3	0	1	0	1	-.005	4
591		11	max	0	10	0	10	.027	1	0	3	0	5	0	10
592			min	-212.686	4	-.311	4	-.02	3	0	1	0	1	-.005	4
593		12	max	0	10	0	10	.027	1	0	3	0	5	0	10
594			min	-212.746	4	-.622	4	-.02	3	0	1	0	1	-.005	4
595		13	max	0	10	0	10	.027	1	0	3	0	5	0	10
596			min	-212.807	4	-.933	4	-.038	5	0	1	0	3	-.004	4
597		14	max	0	10	0	10	.027	1	0	3	0	4	0	10
598			min	-212.867	4	-1.244	4	-.06	5	0	1	0	3	-.004	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	10	0	10	.027	1	0	3	0	4	0	10
600		min	-212.927	4	-1.555	4	-.083	5	0	1	0	3	-.003	4
601	16	max	0	10	0	10	.027	1	0	3	0	4	0	10
602		min	-212.988	4	-1.866	4	-.106	5	0	1	0	3	-.003	4
603	17	max	0	10	0	10	.027	1	0	3	0	1	0	10
604		min	-213.048	4	-2.177	4	-.128	5	0	1	0	3	-.002	4
605	18	max	.015	2	0	10	.027	1	0	3	0	1	0	10
606		min	-213.109	4	-2.488	4	-.151	5	0	1	0	5	-.001	4
607	19	max	.095	2	0	10	.027	1	0	3	0	1	0	1
608		min	-213.169	4	-2.799	4	-.173	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.003	1	.007	2	.009	1	1.507e-3	5	NC	3	NC	2
2			min	-.003	3	-.006	3	-.014	5	-6.723e-4	1	4957.802	2	3553.315	1
3		2	max	.002	1	.006	2	.009	1	1.529e-3	5	NC	3	NC	2
4			min	-.003	3	-.006	3	-.014	5	-6.453e-4	1	5391.368	2	3846.571	1
5		3	max	.002	1	.006	2	.008	1	1.551e-3	5	NC	3	NC	2
6			min	-.003	3	-.006	3	-.013	5	-6.182e-4	1	5903.705	2	4192.164	1
7		4	max	.002	1	.005	2	.007	1	1.572e-3	5	NC	1	NC	2
8			min	-.002	3	-.005	3	-.012	5	-5.912e-4	1	6513.401	2	4602.853	1
9		5	max	.002	1	.005	2	.007	1	1.594e-3	5	NC	1	NC	2
10			min	-.002	3	-.005	3	-.012	5	-5.642e-4	1	7245.111	2	5095.681	1
11		6	max	.002	1	.004	2	.006	1	1.616e-3	5	NC	1	NC	2
12			min	-.002	3	-.005	3	-.011	5	-5.371e-4	1	8132.083	2	5693.841	1
13		7	max	.002	1	.004	2	.005	1	1.638e-3	5	NC	1	NC	2
14			min	-.002	3	-.005	3	-.011	5	-5.101e-4	1	9220.011	2	6429.558	1
15		8	max	.002	1	.003	2	.005	1	1.66e-3	5	NC	1	NC	2
16			min	-.002	3	-.004	3	-.01	5	-4.831e-4	1	NC	1	7348.7	1
17		9	max	.001	1	.003	2	.004	1	1.682e-3	5	NC	1	NC	2
18			min	-.002	3	-.004	3	-.009	5	-4.56e-4	1	NC	1	8518.419	1
19		10	max	.001	1	.002	2	.003	1	1.704e-3	5	NC	1	NC	1
20		min	-.001	3	-.004	3	-.008	5	-4.29e-4	1	NC	1	NC	1	
21	11	max	.001	1	.002	2	.003	1	1.725e-3	5	NC	1	NC	1	
22		min	-.001	3	-.003	3	-.008	5	-4.02e-4	1	NC	1	NC	1	
23	12	max	.001	1	.002	2	.002	1	1.747e-3	5	NC	1	NC	1	
24		min	-.001	3	-.003	3	-.007	5	-3.749e-4	1	NC	1	NC	1	
25	13	max	0	1	.001	2	.002	1	1.769e-3	5	NC	1	NC	1	
26		min	0	3	-.003	3	-.006	5	-3.479e-4	1	NC	1	NC	1	
27	14	max	0	1	0	2	.001	1	1.791e-3	5	NC	1	NC	1	
28		min	0	3	-.002	3	-.005	5	-3.209e-4	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	1	1.813e-3	5	NC	1	NC	1	
30		min	0	3	-.002	3	-.004	5	-2.938e-4	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.835e-3	5	NC	1	NC	1	
32		min	0	3	-.001	3	-.003	5	-2.668e-4	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.856e-3	5	NC	1	NC	1	
34		min	0	3	0	3	-.002	5	-2.398e-4	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.878e-3	5	NC	1	NC	1	
36		min	0	3	0	3	-.001	5	-2.127e-4	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.9e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-1.857e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	8.526e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-8.736e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.005	5	1.065e-4	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-8.81e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.009	5	1.278e-4	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-8.884e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.014	5	1.491e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-8.957e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.018	5	1.704e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-9.031e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.023	4	1.917e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-9.104e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.028	4	2.13e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-9.178e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.032	4	2.343e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-9.252e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.037	4	2.556e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	1	-9.325e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.041	4	2.769e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-9.399e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.046	4	2.982e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-9.473e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.05	4	3.195e-4	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-9.546e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.054	4	3.408e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-9.62e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.058	4	3.62e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-9.694e-4	5	NC	1	NC	1
67		15	max	0	3	.005	2	.062	4	3.833e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	12	-9.767e-4	5	9851.229	2	NC	1
69		16	max	0	3	.006	2	.066	4	4.046e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	12	-9.841e-4	5	8295.427	2	NC	1
71		17	max	0	3	.006	2	.07	4	4.259e-4	1	NC	3	NC	1
72			min	0	2	-.007	3	0	12	-9.914e-4	5	7102.038	2	NC	1
73		18	max	0	3	.007	2	.074	4	4.472e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	12	-9.988e-4	5	6175.267	2	NC	1
75		19	max	0	3	.008	2	.078	4	4.685e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	12	-1.006e-3	5	5448.539	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	4.058e-3	5	NC	1	NC	2
78			min	0	3	-.006	3	-.083	4	-5.786e-4	1	NC	1	233.06	4
79		2	max	.002	1	.007	2	0	10	4.058e-3	5	NC	1	NC	2
80			min	0	3	-.006	3	-.076	4	-5.786e-4	1	NC	1	254.063	4
81		3	max	.002	1	.007	2	0	10	4.058e-3	5	NC	1	NC	2
82			min	0	3	-.006	3	-.069	4	-5.786e-4	1	NC	1	279.061	4
83		4	max	.002	1	.006	2	0	10	4.058e-3	5	NC	1	NC	2
84			min	0	3	-.005	3	-.063	4	-5.786e-4	1	NC	1	309.107	4
85		5	max	.002	1	.006	2	0	10	4.058e-3	5	NC	1	NC	2
86			min	0	3	-.005	3	-.056	4	-5.786e-4	1	NC	1	345.633	4
87		6	max	.002	1	.006	2	0	10	4.058e-3	5	NC	1	NC	2
88			min	0	3	-.004	3	-.049	4	-5.786e-4	1	NC	1	390.633	4
89		7	max	.001	1	.005	2	0	10	4.058e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.043	4	-5.786e-4	1	NC	1	446.947	4
91		8	max	.001	1	.005	2	0	10	4.058e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.037	4	-5.786e-4	1	NC	1	518.732	4
93		9	max	.001	1	.004	2	0	10	4.058e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.032	4	-5.786e-4	1	NC	1	612.275	4
95		10	max	.001	1	.004	2	0	10	4.058e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.026	4	-5.786e-4	1	NC	1	737.46	4
97		11	max	0	1	.003	2	0	10	4.058e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.021	4	-5.786e-4	1	NC	1	910.567	4
99		12	max	0	1	.003	2	0	10	4.058e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.002	3	-.017	4	-5.786e-4	1	NC	1	1159.99	4
101		13	max	0	1	.003	2	0	10	4.058e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.013	4	-5.786e-4	1	NC	1	1538.992	4
103		14	max	0	1	.002	2	0	10	4.058e-3	5	NC	1	NC	1
104			min	0	3	-.002	3	-.009	4	-5.786e-4	1	NC	1	2157.436	4
105		15	max	0	1	.002	2	0	10	4.058e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.006	4	-5.786e-4	1	NC	1	3273.426	4
107		16	max	0	1	.001	2	0	10	4.058e-3	5	NC	1	NC	1
108			min	0	3	-.001	3	-.003	4	-5.786e-4	1	NC	1	5620.694	4
109		17	max	0	1	0	2	0	10	4.058e-3	5	NC	1	NC	1
110			min	0	3	0	3	-.002	4	-5.786e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	4.058e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-5.786e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	4.058e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-5.786e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.024	2	.004	1	1.633e-3	4	NC	3	NC	2
116			min	-.01	3	-.019	3	-.014	5	-6.118e-8	10	1382.269	2	8751.789	1
117		2	max	.008	1	.023	2	.004	1	1.653e-3	4	NC	3	NC	2
118			min	-.009	3	-.018	3	-.014	5	-5.787e-8	10	1475.854	2	9446.255	1
119		3	max	.007	1	.021	2	.003	1	1.672e-3	4	NC	3	NC	1
120			min	-.009	3	-.017	3	-.013	5	-5.457e-8	10	1582.69	2	NC	1
121		4	max	.007	1	.019	2	.003	1	1.692e-3	4	NC	3	NC	1
122			min	-.008	3	-.016	3	-.012	5	-5.126e-8	10	1705.426	2	NC	1
123		5	max	.007	1	.018	2	.003	1	1.711e-3	4	NC	3	NC	1
124			min	-.007	3	-.015	3	-.012	5	-4.137e-7	2	1847.476	2	NC	1
125		6	max	.006	1	.017	2	.002	1	1.73e-3	4	NC	3	NC	1
126			min	-.007	3	-.014	3	-.011	5	-2.485e-6	2	2013.309	2	NC	1
127		7	max	.006	1	.015	2	.002	1	1.75e-3	4	NC	3	NC	1
128			min	-.006	3	-.013	3	-.011	5	-4.556e-6	2	2208.885	2	NC	1
129		8	max	.005	1	.014	2	.002	1	1.769e-3	4	NC	3	NC	1
130			min	-.006	3	-.012	3	-.01	5	-6.627e-6	2	2442.338	2	NC	1
131		9	max	.005	1	.012	2	.002	1	1.789e-3	4	NC	3	NC	1
132			min	-.005	3	-.011	3	-.009	5	-8.698e-6	2	2725.06	2	NC	1
133		10	max	.004	1	.011	2	.001	1	1.808e-3	4	NC	3	NC	1
134			min	-.005	3	-.01	3	-.009	5	-1.172e-5	11	3073.506	2	NC	1
135		11	max	.004	1	.009	2	.001	1	1.828e-3	4	NC	3	NC	1
136			min	-.004	3	-.009	3	-.008	5	-1.518e-5	1	3512.364	2	NC	1
137		12	max	.003	1	.008	2	0	1	1.847e-3	4	NC	3	NC	1
138			min	-.004	3	-.008	3	-.007	5	-2.211e-5	1	4080.426	2	NC	1
139		13	max	.003	1	.007	2	0	1	1.866e-3	4	NC	3	NC	1
140			min	-.003	3	-.007	3	-.006	5	-2.905e-5	1	4842.348	2	NC	1
141		14	max	.002	1	.006	2	0	1	1.886e-3	4	NC	3	NC	1
142			min	-.003	3	-.006	3	-.005	5	-3.598e-5	1	5914.504	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.905e-3	4	NC	3	NC	1
144			min	-.002	3	-.005	3	-.004	5	-4.292e-5	1	7529.629	2	NC	1
145		16	max	.001	1	.003	2	0	1	1.925e-3	4	NC	1	NC	1
146			min	-.002	3	-.003	3	-.003	5	-4.985e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.944e-3	4	NC	1	NC	1
148			min	-.001	3	-.002	3	-.002	5	-5.679e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.964e-3	4	NC	1	NC	1
150			min	0	3	-.001	3	-.001	5	-6.372e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.983e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-7.066e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.204e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.118e-4	4	NC	1	NC	1
155		2	max	0	3	.001	2	.005	4	2.708e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-9.039e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.01	4	2.213e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-8.959e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.014	4	1.717e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-8.88e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.019	4	1.221e-5	1	NC	1	NC	1
162			min	0	2	-.007	3	0	1	-8.801e-4	4	8719.243	2	NC	1
163		6	max	0	3	.007	2	.024	4	1.287e-5	3	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	-8.722e-4	4	6987.727	2	NC	1
165		7	max	0	3	.008	2	.029	4	2.71e-5	3	NC	3	NC	1
166			min	-.001	2	-.01	3	0	1	-8.643e-4	4	5803.399	2	NC	1
167		8	max	.001	3	.009	2	.034	4	4.132e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	-.001	1	-8.564e-4	4	4935.04	2	NC	1
169		9	max	.001	3	.011	2	.038	4	5.555e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	-.001	1	-8.485e-4	4	4267.451	2	NC	1
171		10	max	.001	3	.012	2	.043	4	6.977e-5	3	NC	3	NC	1
172			min	-.002	2	-.014	3	-.001	1	-8.406e-4	4	3736.779	2	NC	1
173		11	max	.002	3	.014	2	.047	4	8.399e-5	3	NC	3	NC	1
174			min	-.002	2	-.015	3	-.001	1	-8.326e-4	4	3304.675	2	NC	1
175		12	max	.002	3	.016	2	.051	4	9.822e-5	3	NC	3	NC	1
176			min	-.002	2	-.016	3	-.002	1	-8.247e-4	4	2946.588	2	NC	1
177		13	max	.002	3	.017	2	.056	4	1.124e-4	3	NC	3	NC	1
178			min	-.003	2	-.017	3	-.002	1	-8.168e-4	4	2645.948	2	NC	1
179		14	max	.002	3	.019	2	.06	4	1.267e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	-.002	1	-8.089e-4	4	2391.069	2	NC	1
181		15	max	.002	3	.021	2	.064	4	1.409e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	-.002	1	-8.01e-4	4	2173.395	2	NC	1
183		16	max	.002	3	.023	2	.068	4	1.551e-4	3	NC	3	NC	1
184			min	-.003	2	-.02	3	-.002	1	-7.931e-4	4	1986.471	2	NC	1
185		17	max	.003	3	.025	2	.072	4	1.693e-4	3	NC	3	NC	1
186			min	-.003	2	-.021	3	-.002	1	-7.852e-4	4	1825.304	2	NC	1
187		18	max	.003	3	.027	2	.076	4	1.836e-4	3	NC	3	NC	1
188			min	-.004	2	-.022	3	-.002	1	-7.773e-4	4	1685.958	2	NC	1
189		19	max	.003	3	.029	2	.08	4	1.978e-4	3	NC	3	NC	1
190			min	-.004	2	-.022	3	-.002	1	-7.694e-4	4	1565.285	2	NC	1
191	M8	1	max	.006	1	.027	2	.002	1	3.85e-3	4	NC	1	NC	2
192			min	-.001	3	-.02	3	-.084	4	-1.543e-4	3	NC	1	230.559	4
193		2	max	.006	1	.026	2	.002	1	3.85e-3	4	NC	1	NC	1
194			min	-.001	3	-.019	3	-.077	4	-1.543e-4	3	NC	1	251.336	4
195		3	max	.005	1	.024	2	.002	1	3.85e-3	4	NC	1	NC	1
196			min	-.001	3	-.018	3	-.07	4	-1.543e-4	3	NC	1	276.065	4
197		4	max	.005	1	.023	2	.001	1	3.85e-3	4	NC	1	NC	1
198			min	-.001	3	-.016	3	-.063	4	-1.543e-4	3	NC	1	305.787	4
199		5	max	.005	1	.021	2	.001	1	3.85e-3	4	NC	1	NC	1
200			min	0	3	-.015	3	-.057	4	-1.543e-4	3	NC	1	341.921	4
201		6	max	.004	1	.02	2	.001	1	3.85e-3	4	NC	1	NC	1
202			min	0	3	-.014	3	-.05	4	-1.543e-4	3	NC	1	386.438	4
203		7	max	.004	1	.018	2	.001	1	3.85e-3	4	NC	1	NC	1
204			min	0	3	-.013	3	-.044	4	-1.543e-4	3	NC	1	442.146	4
205		8	max	.004	1	.017	2	0	1	3.85e-3	4	NC	1	NC	1
206			min	0	3	-.012	3	-.038	4	-1.543e-4	3	NC	1	513.159	4
207		9	max	.003	1	.015	2	0	1	3.85e-3	4	NC	1	NC	1
208			min	0	3	-.011	3	-.032	4	-1.543e-4	3	NC	1	605.697	4
209		10	max	.003	1	.014	2	0	1	3.85e-3	4	NC	1	NC	1
210			min	0	3	-.01	3	-.026	4	-1.543e-4	3	NC	1	729.538	4
211		11	max	.003	1	.012	2	0	1	3.85e-3	4	NC	1	NC	1
212			min	0	3	-.009	3	-.021	4	-1.543e-4	3	NC	1	900.785	4
213		12	max	.002	1	.011	2	0	1	3.85e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.007	4	3.173e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-8.395e-4	4	NC	1	6366.069	4
273		4	max	0	3	0	2	.011	4	1.685e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-9.195e-4	4	NC	1	4210.049	4
275		5	max	0	3	0	2	.015	4	1.972e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-9.994e-4	4	NC	1	3137.259	4
277		6	max	0	3	0	2	.018	4	-8.541e-6	12	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-1.079e-3	4	NC	1	2497.118	4
279		7	max	0	3	0	2	.022	5	-1.617e-5	10	NC	1	NC	1
280			min	0	2	-.004	3	-.001	3	-1.159e-3	4	NC	1	2069.536	5
281		8	max	0	3	0	2	.026	5	-1.816e-5	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-1.239e-3	4	NC	1	1764.597	5
283		9	max	0	3	.001	2	.03	5	-2.014e-5	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	1	-1.319e-3	4	NC	1	1537.405	5
285		10	max	0	3	.001	2	.034	5	-2.213e-5	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	1	-1.399e-3	4	NC	1	1361.749	5
287		11	max	0	3	.002	2	.038	5	-2.411e-5	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	1	-1.479e-3	4	NC	1	1221.913	5
289		12	max	0	3	.002	2	.042	5	-2.61e-5	10	NC	1	NC	1
290			min	0	2	-.007	3	-.004	1	-1.559e-3	4	NC	1	1107.907	5
291		13	max	0	3	.003	2	.045	5	-2.809e-5	10	NC	1	NC	2
292			min	0	2	-.007	3	-.005	1	-1.639e-3	4	NC	1	1013.072	5
293		14	max	0	3	.004	2	.049	5	-3.007e-5	10	NC	1	NC	2
294			min	0	2	-.007	3	-.005	1	-1.719e-3	4	NC	1	932.806	5
295		15	max	0	3	.005	2	.053	5	-3.206e-5	10	NC	1	NC	2
296			min	0	2	-.007	3	-.006	1	-1.799e-3	4	9867.28	2	863.822	5
297		16	max	0	3	.006	2	.057	5	-3.404e-5	10	NC	1	NC	2
298			min	0	2	-.007	3	-.007	1	-1.879e-3	4	8307.489	2	803.716	5
299		17	max	0	3	.006	2	.061	5	-3.603e-5	10	NC	3	NC	2
300			min	0	2	-.007	3	-.008	1	-1.959e-3	4	7111.374	2	750.686	5
301		18	max	0	3	.007	2	.065	5	-3.802e-5	10	NC	3	NC	2
302			min	0	2	-.007	3	-.008	1	-2.039e-3	4	6182.697	2	703.361	5
303		19	max	0	3	.008	2	.07	5	-4.e-5	10	NC	3	NC	2
304			min	0	2	-.007	3	-.009	1	-2.119e-3	4	5454.611	2	660.68	5
305	M12	1	max	.002	1	.008	2	.007	1	5.005e-3	4	NC	1	NC	3
306			min	0	3	-.006	3	-.077	5	4.047e-5	10	NC	1	251.705	5
307		2	max	.002	1	.007	2	.007	1	5.005e-3	4	NC	1	NC	3
308			min	0	3	-.006	3	-.07	5	4.047e-5	10	NC	1	274.382	5
309		3	max	.002	1	.007	2	.006	1	5.005e-3	4	NC	1	NC	3
310			min	0	3	-.006	3	-.064	5	4.047e-5	10	NC	1	301.373	5
311		4	max	.002	1	.006	2	.006	1	5.005e-3	4	NC	1	NC	2
312			min	0	3	-.005	3	-.058	5	4.047e-5	10	NC	1	333.813	5
313		5	max	.002	1	.006	2	.005	1	5.005e-3	4	NC	1	NC	2
314			min	0	3	-.005	3	-.052	5	4.047e-5	10	NC	1	373.25	5
315		6	max	.002	1	.006	2	.004	1	5.005e-3	4	NC	1	NC	2
316			min	0	3	-.004	3	-.046	5	4.047e-5	10	NC	1	421.836	5
317		7	max	.001	1	.005	2	.004	1	5.005e-3	4	NC	1	NC	2
318			min	0	3	-.004	3	-.04	5	4.047e-5	10	NC	1	482.636	5
319		8	max	.001	1	.005	2	.003	1	5.005e-3	4	NC	1	NC	2
320			min	0	3	-.004	3	-.035	5	4.047e-5	10	NC	1	560.137	5
321		9	max	.001	1	.004	2	.003	1	5.005e-3	4	NC	1	NC	2
322			min	0	3	-.003	3	-.029	5	4.047e-5	10	NC	1	661.129	5
323		10	max	.001	1	.004	2	.002	1	5.005e-3	4	NC	1	NC	2
324			min	0	3	-.003	3	-.024	5	4.047e-5	10	NC	1	796.28	5
325		11	max	0	1	.003	2	.002	1	5.005e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.02	5	4.047e-5	10	NC	1	983.166	5
327		12	max	0	1	.003	2	.001	1	5.005e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.002	3	-.015	5	4.047e-5	10	NC	1	1252.439	5
329		max	0	1	.003	2	.001	1	5.005e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.012	5	4.047e-5	10	NC	1	1661.597	5
331		max	0	1	.002	2	0	1	5.005e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.008	5	4.047e-5	10	NC	1	2329.238	5
333		max	0	1	.002	2	0	1	5.005e-3	4	NC	1	NC	1
334		min	0	3	-.001	3	-.005	5	4.047e-5	10	NC	1	3533.986	5
335		max	0	1	.001	2	0	1	5.005e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.003	5	4.047e-5	10	NC	1	6067.897	5
337		max	0	1	0	2	0	1	5.005e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.001	5	4.047e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	5.005e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	4.047e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	5.005e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	4.047e-5	10	NC	1	NC	1
343	M1	max	.006	3	.022	3	.007	5	1.719e-2	1	NC	1	NC	1
344		min	-.007	2	-.023	1	-.004	1	-1.83e-2	3	NC	1	NC	1
345		max	.006	3	.012	3	.01	5	8.301e-3	1	NC	4	NC	1
346		min	-.007	2	-.012	1	-.007	1	-9.047e-3	3	4507.234	1	NC	1
347		max	.006	3	.003	3	.014	5	3.87e-4	5	NC	4	NC	2
348		min	-.007	2	-.003	1	-.009	1	-4.209e-4	1	2325.107	1	6667.491	5
349		max	.006	3	.006	1	.018	5	3.866e-4	5	NC	4	NC	2
350		min	-.007	2	-.004	3	-.011	1	-3.541e-4	1	1642.947	1	4230.648	5
351		max	.006	3	.013	1	.023	5	3.862e-4	5	NC	5	NC	2
352		min	-.007	2	-.01	3	-.011	1	-2.874e-4	1	1315.087	1	3040.198	5
353		max	.006	3	.018	1	.027	5	3.858e-4	5	NC	5	NC	2
354		min	-.007	2	-.015	3	-.01	1	-2.206e-4	1	1129.692	1	2343.669	5
355		max	.006	3	.023	1	.032	5	3.854e-4	5	NC	5	NC	2
356		min	-.007	2	-.019	3	-.009	1	-1.539e-4	1	1017.326	1	1891.523	5
357		max	.006	3	.026	1	.037	5	3.85e-4	5	NC	5	NC	1
358		min	-.007	2	-.021	3	-.007	1	-8.711e-5	1	949.116	1	1577.367	5
359		max	.006	3	.028	1	.042	5	3.845e-4	5	NC	5	NC	1
360		min	-.007	2	-.023	3	-.005	1	-2.036e-5	1	911.575	1	1343.084	4
361		max	.006	3	.028	1	.047	5	3.946e-4	4	NC	5	NC	1
362		min	-.007	2	-.023	3	-.003	1	1.242e-5	10	898.467	1	1156.929	4
363		max	.006	3	.028	1	.052	4	4.084e-4	4	NC	5	NC	1
364		min	-.007	2	-.022	3	0	1	1.666e-5	10	907.869	1	1015.471	4
365		max	.006	3	.026	2	.058	4	4.223e-4	4	NC	5	NC	1
366		min	-.007	2	-.02	3	0	10	2.091e-5	10	940.881	2	905.611	4
367		max	.006	3	.023	2	.063	4	4.361e-4	4	NC	5	NC	2
368		min	-.007	2	-.018	3	0	10	2.515e-5	10	998.151	2	818.857	4
369		max	.006	3	.018	2	.069	4	4.499e-4	4	NC	5	NC	2
370		min	-.007	2	-.014	3	0	10	2.939e-5	10	1097.446	2	749.507	4
371		max	.006	3	.012	2	.074	4	4.637e-4	4	NC	5	NC	2
372		min	-.007	2	-.009	3	0	12	3.076e-5	12	1265.676	2	693.617	4
373		max	.006	3	.005	2	.079	4	7.384e-4	4	NC	4	NC	2
374		min	-.007	2	-.004	3	0	12	3.117e-5	12	1568.07	2	648.393	4
375		max	.006	3	.002	3	.083	4	7.221e-3	4	NC	4	NC	2
376		min	-.007	2	-.004	2	0	10	1.549e-5	10	2208.526	2	611.857	4
377		max	.006	3	.009	3	.087	4	9.861e-3	1	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-4.246e-3	3	4270.442	2	582.476	4
379		max	.006	3	.016	3	.09	4	1.987e-2	1	NC	1	NC	1
380		min	-.007	2	-.026	2	-.002	1	-8.605e-3	3	NC	1	559.801	4
381	M5	max	.018	3	.071	3	.007	5	6.869e-6	4	NC	1	NC	1
382		min	-.024	2	-.076	1	-.004	1	5.31e-8	2	NC	1	NC	1
383		max	.018	3	.039	3	.01	5	1.886e-4	5	NC	5	NC	1
384		min	-.024	2	-.041	1	-.004	1	-7.396e-5	1	1331.205	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.018	3	.01	3	.014	5	3.673e-4	5	NC	5	NC	1
386			min	-.024	2	-.009	1	-.004	1	-1.466e-4	1	686.031	1	NC	1
387		4	max	.018	3	.019	1	.019	5	3.82e-4	5	NC	5	NC	1
388			min	-.024	2	-.014	3	-.004	1	-1.393e-4	1	483.851	1	NC	1
389		5	max	.018	3	.043	1	.023	5	3.968e-4	5	NC	5	NC	1
390			min	-.024	2	-.034	3	-.003	1	-1.32e-4	1	386.551	1	NC	1
391		6	max	.018	3	.063	1	.028	5	4.115e-4	5	NC	5	NC	1
392			min	-.024	2	-.05	3	-.003	1	-1.247e-4	1	331.426	1	NC	1
393		7	max	.018	3	.078	1	.033	5	4.262e-4	5	NC	15	NC	1
394			min	-.024	2	-.062	3	-.003	1	-1.174e-4	1	297.911	1	NC	1
395		8	max	.018	3	.089	1	.039	5	4.409e-4	5	NC	15	NC	1
396			min	-.024	2	-.07	3	-.003	1	-1.101e-4	1	277.445	1	NC	1
397		9	max	.018	3	.096	1	.044	5	4.556e-4	5	NC	15	NC	1
398			min	-.024	2	-.074	3	-.003	1	-1.028e-4	1	266.023	1	NC	1
399		10	max	.018	3	.098	1	.05	5	4.703e-4	5	NC	15	NC	1
400			min	-.024	2	-.075	3	-.003	1	-9.554e-5	1	261.784	1	NC	1
401		11	max	.018	3	.096	1	.055	5	4.85e-4	5	NC	15	NC	1
402			min	-.024	2	-.072	3	-.003	1	-8.825e-5	1	264.138	1	NC	1
403		12	max	.018	3	.089	1	.06	4	4.997e-4	5	NC	15	NC	1
404			min	-.024	2	-.066	3	-.002	1	-8.095e-5	1	273.531	1	NC	1
405		13	max	.018	3	.078	1	.066	4	5.144e-4	5	NC	15	NC	1
406			min	-.024	2	-.057	3	-.002	1	-7.366e-5	1	291.642	1	NC	1
407		14	max	.018	3	.062	1	.071	4	5.291e-4	5	NC	5	NC	1
408			min	-.024	2	-.045	3	-.002	1	-6.636e-5	1	322.194	1	NC	1
409		15	max	.018	3	.042	1	.075	4	5.438e-4	5	NC	5	NC	1
410			min	-.024	2	-.03	3	-.002	1	-5.906e-5	1	373.218	1	NC	1
411		16	max	.018	3	.017	1	.08	4	8.155e-4	5	NC	5	NC	1
412			min	-.024	2	-.013	3	-.002	1	-5.578e-5	1	464.039	1	NC	1
413		17	max	.018	3	.008	3	.084	4	7.242e-3	4	NC	5	NC	1
414			min	-.024	2	-.014	2	-.002	1	-1.479e-4	1	654.126	1	NC	1
415		18	max	.018	3	.03	3	.087	4	3.716e-3	4	NC	5	NC	1
416			min	-.024	2	-.049	2	-.002	1	-7.553e-5	1	1265.725	1	NC	1
417		19	max	.018	3	.053	3	.09	4	2.468e-6	5	NC	1	NC	1
418			min	-.024	2	-.087	2	-.002	1	-2.102e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.021	3	.006	5	1.83e-2	3	NC	1	NC	1
420			min	-.007	2	-.023	1	-.005	1	-1.719e-2	1	NC	1	NC	1
421		2	max	.006	3	.012	3	.005	5	9.068e-3	3	NC	4	NC	1
422			min	-.007	2	-.012	1	0	1	-8.479e-3	1	4508.518	1	NC	1
423		3	max	.006	3	.003	3	.006	4	6.889e-5	1	NC	4	NC	2
424			min	-.007	2	-.003	1	0	3	-1.598e-5	5	2325.789	1	8499.025	1
425		4	max	.006	3	.006	1	.007	4	2.311e-5	2	NC	4	NC	2
426			min	-.007	2	-.004	3	0	3	-2.704e-5	5	1643.427	1	7080.685	1
427		5	max	.006	3	.013	1	.009	4	8.939e-6	10	NC	5	NC	2
428			min	-.007	2	-.011	3	-.001	3	-4.605e-5	4	1315.45	1	6858.181	1
429		6	max	.006	3	.018	1	.012	4	4.688e-6	10	NC	5	NC	2
430			min	-.007	2	-.015	3	-.002	3	-9.308e-5	1	1129.982	1	6001.502	4
431		7	max	.006	3	.023	1	.016	4	4.381e-7	10	NC	5	NC	2
432			min	-.007	2	-.019	3	-.002	3	-1.471e-4	1	1017.563	1	4080.155	4
433		8	max	.006	3	.026	1	.02	4	-3.812e-6	10	NC	5	NC	1
434			min	-.007	2	-.022	3	-.003	3	-2.011e-4	1	949.314	1	2974.116	4
435		9	max	.006	3	.028	1	.025	4	-8.062e-6	10	NC	5	NC	1
436			min	-.007	2	-.023	3	-.003	3	-2.551e-4	1	911.743	1	2279.066	4
437		10	max	.006	3	.028	1	.031	5	-1.231e-5	10	NC	5	NC	1
438			min	-.007	2	-.023	3	-.003	1	-3.09e-4	1	898.61	1	1813.414	4
439		11	max	.006	3	.028	1	.037	5	-1.656e-5	10	NC	5	NC	1
440			min	-.007	2	-.022	3	-.005	1	-3.63e-4	1	907.989	1	1485.9	4
441		12	max	.006	3	.026	2	.043	5	-2.081e-5	10	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.007	2	-.02	3	-.007	1	-4.17e-4	1	941.334	2	1246.599	4
443		13	max	.006	3	.023	2	.05	5	-2.506e-5	10	NC	5	NC	2
444			min	-.007	2	-.018	3	-.008	1	-4.71e-4	1	998.62	2	1065.527	5
445		14	max	.006	3	.018	2	.056	5	-2.931e-5	10	NC	5	NC	2
446			min	-.007	2	-.014	3	-.009	1	-5.25e-4	1	1097.948	2	924.196	5
447		15	max	.006	3	.012	2	.063	5	-3.356e-5	10	NC	5	NC	2
448			min	-.007	2	-.009	3	-.009	1	-5.79e-4	1	1266.237	2	814.99	5
449		16	max	.006	3	.005	2	.07	5	1.207e-4	5	NC	4	NC	2
450			min	-.007	2	-.004	3	-.009	1	-6.2e-4	1	1568.738	2	729.039	5
451		17	max	.006	3	.002	3	.077	5	7.064e-3	5	NC	4	NC	2
452			min	-.007	2	-.004	2	-.007	1	-3.509e-4	1	2209.402	2	657.411	4
453		18	max	.006	3	.009	3	.083	5	4.259e-3	3	NC	4	NC	1
454			min	-.007	2	-.015	2	-.005	1	-1.001e-2	1	4272.078	2	597.944	4
455		19	max	.006	3	.016	3	.09	4	8.604e-3	3	NC	1	NC	1
456			min	-.007	2	-.026	2	-.001	1	-1.987e-2	1	NC	1	548.734	4
457	M13	1	max	.005	1	.021	3	.006	3	3.633e-3	3	NC	1	NC	1
458			min	-.006	5	-.023	1	-.007	2	-3.893e-3	1	NC	1	NC	1
459		2	max	.004	1	.169	3	.019	1	4.549e-3	3	NC	5	NC	2
460			min	-.006	5	-.161	1	-.002	10	-4.921e-3	1	1018.578	3	6480.253	1
461		3	max	.004	1	.289	3	.05	1	5.465e-3	3	NC	5	NC	3
462			min	-.006	5	-.274	1	-.003	5	-5.948e-3	1	559.983	3	2769.704	1
463		4	max	.004	1	.365	3	.076	1	6.38e-3	3	NC	5	NC	3
464			min	-.006	5	-.346	1	-.005	5	-6.976e-3	1	436.272	3	1881.851	1
465		5	max	.004	1	.388	3	.087	1	7.296e-3	3	NC	5	NC	3
466			min	-.006	5	-.369	1	-.008	5	-8.004e-3	1	409.131	3	1642.122	1
467		6	max	.004	1	.359	3	.081	1	8.212e-3	3	NC	5	NC	3
468			min	-.006	5	-.342	1	-.01	5	-9.031e-3	1	444.756	3	1754.867	1
469		7	max	.004	1	.288	3	.059	1	9.128e-3	3	NC	5	NC	3
470			min	-.006	5	-.277	1	-.011	5	-1.006e-2	1	563.555	3	2366.746	1
471		8	max	.004	1	.195	3	.027	1	1.004e-2	3	NC	5	NC	2
472			min	-.007	5	-.191	1	-.011	5	-1.109e-2	1	865.719	3	4809.033	1
473		9	max	.004	1	.11	3	.017	3	1.096e-2	3	NC	5	NC	1
474			min	-.007	5	-.112	1	-.018	2	-1.211e-2	1	1674.854	1	NC	1
475		10	max	.004	1	.071	3	.018	3	1.187e-2	3	NC	4	NC	1
476			min	-.007	5	-.076	1	-.024	2	-1.314e-2	1	2800.723	1	8804.989	2
477		11	max	.004	1	.11	3	.021	3	1.096e-2	3	NC	5	NC	1
478			min	-.007	5	-.112	1	-.017	2	-1.211e-2	1	1674.855	1	NC	1
479		12	max	.004	1	.195	3	.028	1	1.004e-2	3	NC	5	NC	2
480			min	-.007	5	-.191	1	-.008	10	-1.109e-2	1	865.719	3	4662.022	1
481		13	max	.004	1	.288	3	.06	1	9.129e-3	3	NC	5	NC	5
482			min	-.007	5	-.277	1	-.004	10	-1.006e-2	1	563.555	3	2329.436	1
483		14	max	.004	1	.359	3	.082	1	8.214e-3	3	NC	5	NC	5
484			min	-.007	5	-.342	1	-.001	10	-9.032e-3	1	444.756	3	1737.575	1
485		15	max	.004	1	.388	3	.088	1	7.299e-3	3	NC	5	NC	3
486			min	-.007	5	-.369	1	0	10	-8.004e-3	1	409.131	3	1631.878	1
487		16	max	.004	1	.365	3	.076	1	6.384e-3	3	NC	5	NC	3
488			min	-.007	5	-.346	1	-.003	5	-6.976e-3	1	436.272	3	1875.579	1
489		17	max	.004	1	.29	3	.05	1	5.469e-3	3	NC	5	NC	3
490			min	-.007	5	-.274	1	-.006	5	-5.949e-3	1	559.983	3	2769.026	1
491		18	max	.004	1	.169	3	.019	1	4.554e-3	3	NC	5	NC	2
492			min	-.007	5	-.161	1	-.006	5	-4.921e-3	1	1018.578	3	6509.308	1
493		19	max	.004	1	.022	3	.006	3	3.638e-3	3	NC	1	NC	1
494			min	-.007	5	-.023	1	-.007	2	-3.893e-3	1	NC	1	NC	1
495	M16	1	max	.001	1	.016	3	.006	3	4.265e-3	2	NC	1	NC	1
496			min	-.09	4	-.026	2	-.007	2	-2.743e-3	3	NC	1	NC	1
497		2	max	.001	1	.086	3	.019	4	5.378e-3	2	NC	5	NC	2
498			min	-.09	4	-.185	1	-.002	10	-3.423e-3	3	938.098	1	6409.902	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.143	3	.05	1	6.49e-3	2	NC	5	NC	3
500		min	-.09	4	-.316	1	0	10	-4.103e-3	3	515.372	1	2748.093	1
501	4	max	.002	1	.18	3	.076	1	7.603e-3	2	NC	5	NC	3
502		min	-.09	4	-.399	1	0	10	-4.782e-3	3	400.998	1	1869.678	1
503	5	max	.002	1	.192	3	.088	1	8.716e-3	2	NC	5	NC	3
504		min	-.09	4	-.425	1	0	10	-5.462e-3	3	375.232	1	1632.53	1
505	6	max	.002	1	.18	3	.082	1	9.829e-3	2	NC	5	NC	5
506		min	-.09	4	-.394	1	-.001	10	-6.142e-3	3	406.368	1	1745.015	1
507	7	max	.002	1	.15	3	.059	1	1.094e-2	2	NC	5	NC	5
508		min	-.09	4	-.318	1	-.004	10	-6.822e-3	3	511.253	1	2353.373	1
509	8	max	.002	1	.109	3	.027	1	1.206e-2	2	NC	5	NC	2
510		min	-.09	4	-.219	1	-.008	10	-7.502e-3	3	773.22	1	4782.55	1
511	9	max	.002	1	.07	3	.02	3	1.317e-2	2	NC	5	NC	1
512		min	-.09	4	-.128	2	-.018	2	-8.181e-3	3	1462.833	1	NC	1
513	10	max	.002	1	.053	3	.018	3	1.428e-2	2	NC	4	NC	1
514		min	-.09	4	-.087	2	-.024	2	-8.861e-3	3	2464.399	1	8732.938	2
515	11	max	.002	1	.07	3	.017	3	1.317e-2	2	NC	5	NC	1
516		min	-.09	4	-.128	2	-.018	2	-8.181e-3	3	1462.834	1	NC	1
517	12	max	.002	1	.109	3	.027	1	1.206e-2	2	NC	5	NC	2
518		min	-.09	4	-.219	1	-.008	10	-7.5e-3	3	773.22	1	4781.584	1
519	13	max	.002	1	.15	3	.059	1	1.094e-2	2	NC	5	NC	3
520		min	-.09	4	-.318	1	-.004	10	-6.82e-3	3	511.253	1	2360.792	1
521	14	max	.002	1	.18	3	.081	1	9.83e-3	2	NC	5	NC	3
522		min	-.09	4	-.394	1	-.001	10	-6.14e-3	3	406.368	1	1754.283	1
523	15	max	.002	1	.192	3	.087	1	8.717e-3	2	NC	5	NC	3
524		min	-.09	4	-.425	1	-.003	5	-5.459e-3	3	375.232	1	1644.806	1
525	16	max	.002	1	.18	3	.075	1	7.605e-3	2	NC	5	NC	3
526		min	-.09	4	-.399	1	-.007	5	-4.779e-3	3	400.998	1	1889.119	1
527	17	max	.002	1	.143	3	.049	1	6.492e-3	2	NC	5	NC	3
528		min	-.09	4	-.316	1	-.009	5	-4.099e-3	3	515.373	1	2789.191	1
529	18	max	.002	1	.086	3	.018	1	5.379e-3	2	NC	5	NC	2
530		min	-.09	4	-.185	1	-.008	5	-3.418e-3	3	938.099	1	6564.817	1
531	19	max	.002	1	.016	3	.006	3	4.267e-3	2	NC	1	NC	1
532		min	-.09	4	-.026	2	-.007	2	-2.738e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.201e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.268e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.007	4	8.146e-4	3	NC	1	NC	1
536		min	0	5	-.008	1	0	3	-6.393e-4	1	NC	1	NC	1
537	3	max	0	3	0	15	.016	4	1.309e-3	3	NC	5	NC	1
538		min	-.001	5	-.015	1	-.003	3	-1.226e-3	1	5486.946	2	5165.406	4
539	4	max	0	3	0	15	.024	4	1.804e-3	3	NC	5	NC	9
540		min	-.002	5	-.022	1	-.007	3	-1.812e-3	1	3764.366	2	3388.85	4
541	5	max	0	3	0	15	.032	4	2.298e-3	3	NC	5	NC	9
542		min	-.003	5	-.029	1	-.011	3	-2.398e-3	1	2937.373	2	2577.737	4
543	6	max	0	3	0	15	.038	4	2.793e-3	3	NC	5	8134.065	9
544		min	-.004	5	-.034	1	-.015	3	-2.984e-3	1	2472.11	2	2148.733	4
545	7	max	0	3	0	15	.043	4	3.287e-3	3	NC	5	6379.553	9
546		min	-.004	5	-.038	1	-.02	3	-3.57e-3	1	2192.315	2	1914.477	4
547	8	max	0	3	0	15	.045	4	3.782e-3	3	NC	5	5273.255	9
548		min	-.005	5	-.042	1	-.025	3	-4.157e-3	1	2024.397	2	1800.779	4
549	9	max	0	3	0	15	.046	4	4.276e-3	3	NC	5	4547.901	9
550		min	-.006	5	-.044	1	-.029	3	-4.743e-3	1	1934.013	2	1777.712	4
551	10	max	0	3	0	15	.044	4	4.771e-3	3	NC	5	4068.796	9
552		min	-.007	5	-.044	1	-.032	3	-5.329e-3	1	1905.42	2	1764.118	1
553	11	max	0	3	0	15	.041	4	5.266e-3	3	NC	5	3765.12	9
554		min	-.007	5	-.044	1	-.035	3	-5.915e-3	1	1934.013	2	1631.91	1
555	12	max	0	3	0	15	.04	1	5.76e-3	3	NC	5	4294.531	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.008	5	-0.042	1	-0.036	3	-6.501e-3	1	2024.397	2	1560.704	1
557	13	max	0	3	0	15	.039	1	6.255e-3	3	NC	5	5604.331	15
558		min	-0.009	5	-0.039	1	-0.035	3	-7.088e-3	1	2192.315	2	1546.169	1
559	14	max	0	3	0	15	.036	1	6.749e-3	3	NC	5	8362.152	15
560		min	-0.009	5	-0.035	1	-0.032	3	-7.674e-3	1	2472.11	2	1595.204	1
561	15	max	0	3	.001	5	.031	1	7.244e-3	3	NC	5	NC	5
562		min	-0.01	5	-0.03	1	-0.028	3	-8.26e-3	1	2937.373	2	1732.725	1
563	16	max	0	3	.002	5	.023	1	7.738e-3	3	NC	5	NC	4
564		min	-0.011	5	-0.024	1	-.02	3	-8.846e-3	1	3764.366	2	2026.219	1
565	17	max	0	3	.003	5	.012	1	8.233e-3	3	NC	5	NC	4
566		min	-0.012	5	-0.017	1	-.01	3	-9.432e-3	1	5486.946	2	2687.281	1
567	18	max	0	3	.004	5	.004	3	8.727e-3	3	NC	1	NC	4
568		min	-0.012	5	-.01	1	-.008	2	-1.002e-2	1	NC	1	4786.146	1
569	19	max	0	3	.005	5	.021	3	9.222e-3	3	NC	1	NC	1
570		min	-0.013	5	-.003	1	-.025	2	-1.06e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	.007	3	2.739e-3	3	NC	1	NC	1
572		min	-0.005	4	-.003	4	-.007	2	-2.837e-3	1	NC	1	NC	1
573	2	max	0	10	-.003	12	.003	1	2.622e-3	3	NC	1	NC	1
574		min	-0.004	4	-.015	4	-.002	5	-2.704e-3	1	6554.316	4	NC	1
575	3	max	0	10	-.007	12	.009	1	2.505e-3	3	NC	12	NC	4
576		min	-0.004	4	-.027	4	-.007	5	-2.57e-3	1	3335.268	4	6095.921	1
577	4	max	0	10	-.011	12	.014	1	2.389e-3	3	7528.731	12	NC	10
578		min	-0.004	4	-.038	4	-.014	5	-2.436e-3	1	2288.189	4	4631.484	1
579	5	max	0	10	-.014	12	.017	1	2.272e-3	3	5874.746	12	NC	10
580		min	-0.004	4	-.048	4	-.022	5	-2.303e-3	1	1785.497	4	3933.69	5
581	6	max	0	10	-.016	12	.019	1	2.155e-3	3	4944.219	12	NC	10
582		min	-0.003	4	-.057	4	-.03	5	-2.169e-3	1	1502.684	4	2785.447	5
583	7	max	0	10	-.018	12	.019	1	2.039e-3	3	4384.631	12	NC	10
584		min	-0.003	4	-.064	4	-.038	5	-2.041e-3	2	1332.61	4	2180.601	5
585	8	max	0	10	-.02	12	.019	1	1.922e-3	3	4048.793	12	NC	10
586		min	-0.003	4	-.069	4	-.045	5	-1.914e-3	2	1230.54	4	1834.899	5
587	9	max	0	10	-.021	12	.018	1	1.805e-3	3	3868.026	12	NC	10
588		min	-0.003	4	-.072	4	-.051	5	-1.787e-3	2	1175.599	4	1634.27	5
589	10	max	0	10	-.021	12	.016	1	1.689e-3	3	3810.839	12	NC	10
590		min	-0.002	4	-.072	4	-.054	5	-1.66e-3	2	1158.219	4	1527.557	5
591	11	max	0	10	-.021	12	.014	1	1.572e-3	3	3868.026	12	NC	10
592		min	-0.002	4	-.071	4	-.056	5	-1.533e-3	2	1175.599	4	1492.059	5
593	12	max	0	10	-.02	12	.012	1	1.455e-3	3	4048.793	12	NC	9
594		min	-0.002	4	-.068	4	-.054	5	-1.406e-3	2	1230.54	4	1521.476	5
595	13	max	0	10	-.019	12	.009	1	1.339e-3	3	4384.631	12	NC	2
596		min	-0.002	4	-.063	4	-.051	5	-1.279e-3	2	1332.61	4	1623.232	5
597	14	max	0	10	-.016	12	.007	1	1.222e-3	3	4944.219	12	NC	1
598		min	-0.001	4	-.055	4	-.045	5	-1.152e-3	2	1502.684	4	1822.984	5
599	15	max	0	10	-.014	12	.004	1	1.105e-3	3	5874.746	12	NC	1
600		min	-0.001	4	-.047	4	-.038	5	-1.025e-3	2	1785.497	4	2181.802	5
601	16	max	0	10	-.011	12	.002	1	9.885e-4	3	7528.731	12	NC	1
602		min	0	4	-.036	4	-.029	5	-8.978e-4	2	2288.189	4	2851.807	5
603	17	max	0	10	-.007	12	0	1	8.718e-4	3	NC	12	NC	1
604		min	0	4	-.025	4	-.019	5	-7.708e-4	2	3335.268	4	4302.938	5
605	18	max	0	10	-.004	12	0	3	8.266e-4	4	NC	1	NC	1
606		min	0	4	-.013	4	-.009	5	-6.437e-4	2	6554.316	4	8919.362	5
607	19	max	0	1	0	1	0	1	8.964e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.167e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.