

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	Height = 1550 mm
Width =	1050 mm	Width = 970 mm
Dead Load =	3.00 psf	Dead Load = 1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	84 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.617 k-ft
$M_z$ =	0.000 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>58%</b>

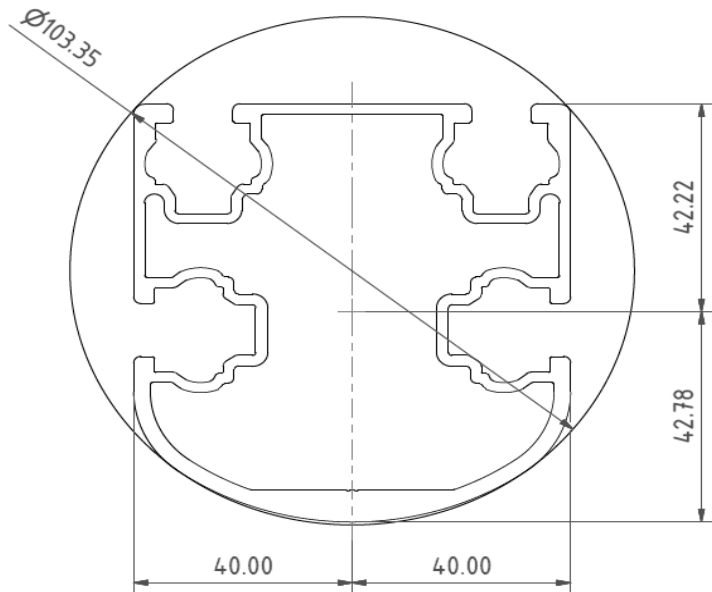


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.633 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.966 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>78%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.402 k-ft
$P_n$ =	0.327 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>30%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.811 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>39%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.908 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>34%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

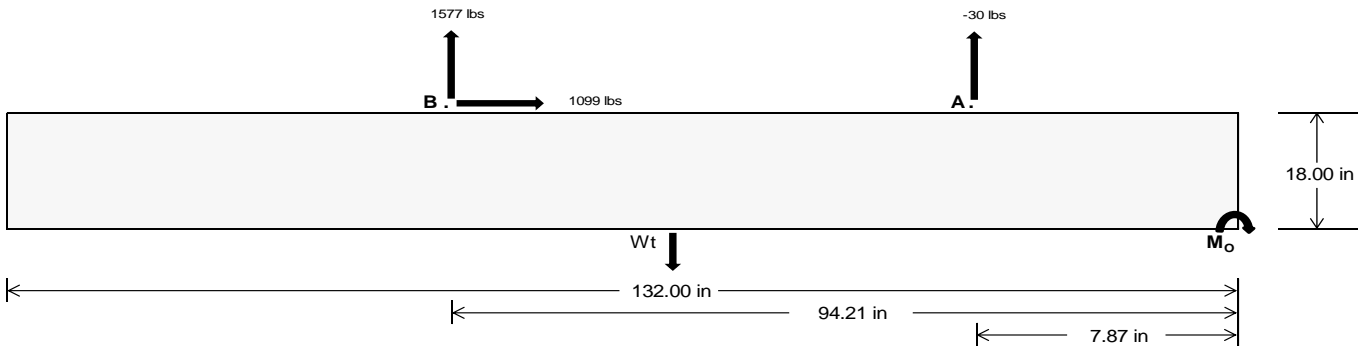
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>99.81</b>	<b>6565.48</b>	k
Compressive Load =	<b>2389.05</b>	<b>4739.66</b>	k
Lateral Load =	<b>289.58</b>	<b>4573.21</b>	k
Moment (Weak Axis) =	<b>0.53</b>	<b>0.14</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 168101.3$  in-lbs  
Resisting Force Required = 2546.99 lbs  
S.F. = 1.67  
Weight Required = 4244.98 lbs  
Minimum Width = 33 in  
Weight Provided = 6579.38 lbs

### Sliding

Force = 1099.37 lbs  
Friction = 0.4  
Weight Required = 2748.44 lbs  
Resisting Weight = 6579.38 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 1099.37 lbs  
Cohesion = 130 psf  
Area = 30.25 ft<sup>2</sup>  
Resisting = 3289.69 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 33in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 33in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
33 in 34 in 35 in 36 in  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.75 \text{ ft}) = 6579 \text{ lbs}$  6779 lbs 6978 lbs 7178 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in
$F_A$	760 lbs	760 lbs	760 lbs	760 lbs	1019 lbs	1019 lbs	1019 lbs	1019 lbs	1238 lbs	1238 lbs	1238 lbs	1238 lbs	61 lbs	61 lbs	61 lbs	61 lbs
$F_B$	667 lbs	667 lbs	667 lbs	667 lbs	2164 lbs	2164 lbs	2164 lbs	2164 lbs	2039 lbs	2039 lbs	2039 lbs	2039 lbs	-3154 lbs	-3154 lbs	-3154 lbs	-3154 lbs
$F_V$	106 lbs	106 lbs	106 lbs	106 lbs	1985 lbs	1985 lbs	1985 lbs	1985 lbs	1556 lbs	1556 lbs	1556 lbs	1556 lbs	-2199 lbs	-2199 lbs	-2199 lbs	-2199 lbs
$P_{total}$	8006 lbs	8206 lbs	8405 lbs	8605 lbs	9763 lbs	9962 lbs	10162 lbs	10361 lbs	9857 lbs	10056 lbs	10256 lbs	10455 lbs	855 lbs	974 lbs	1094 lbs	1214 lbs
$M$	2270 lbs-ft	2270 lbs-ft	2270 lbs-ft	2270 lbs-ft	2826 lbs-ft	2826 lbs-ft	2826 lbs-ft	2826 lbs-ft	3538 lbs-ft	3538 lbs-ft	3538 lbs-ft	3538 lbs-ft	4410 lbs-ft	4410 lbs-ft	4410 lbs-ft	4410 lbs-ft
$e$	0.28 ft	0.28 ft	0.27 ft	0.26 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.36 ft	0.35 ft	0.34 ft	0.34 ft	5.16 ft	4.53 ft	4.03 ft	3.63 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	223.7 psf	223.6 psf	223.4 psf	223.2 psf	271.8 psf	270.2 psf	268.7 psf	267.3 psf	262.1 psf	260.7 psf	259.5 psf	258.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	305.6 psf	303.0 psf	300.6 psf	298.3 psf	373.7 psf	369.1 psf	364.8 psf	360.7 psf	389.6 psf	384.6 psf	379.8 psf	375.3 psf	608.2 psf	235.4 psf	170.3 psf	144.5 psf

Maximum Bearing Pressure = 608 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

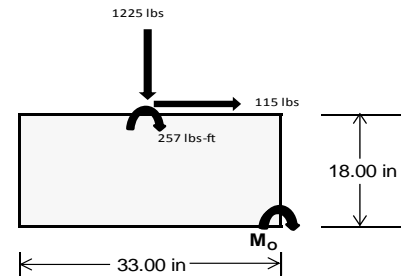
### Overturning Check

$M_o = 1254.5 \text{ ft-lbs}$   
 Resisting Force Required = 912.38 lbs  
 S.F. = 1.67  
 Weight Required = 1520.63 lbs  
 Minimum Width = **33 in**  
 Weight Provided = 6579.38 lbs

*A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	33 in			33 in			33 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	258 lbs	446 lbs	133 lbs	526 lbs	1225 lbs	431 lbs	119 lbs	131 lbs	-5 lbs
$F_v$	158 lbs	155 lbs	161 lbs	117 lbs	115 lbs	123 lbs	159 lbs	155 lbs	160 lbs
$P_{total}$	8403 lbs	8592 lbs	8279 lbs	8280 lbs	8979 lbs	8185 lbs	2501 lbs	2512 lbs	2377 lbs
$M$	581 lbs-ft	571 lbs-ft	589 lbs-ft	432 lbs-ft	430 lbs-ft	452 lbs-ft	582 lbs-ft	571 lbs-ft	584 lbs-ft
$e$	0.07 ft	0.07 ft	0.07 ft	0.05 ft	0.05 ft	0.06 ft	0.23 ft	0.23 ft	0.25 ft
$L/6$	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft
$f_{min}$	235.9 psf	242.8 psf	231.2 psf	242.5 psf	265.8 psf	238.0 psf	40.7 psf	41.9 psf	36.4 psf
$f_{max}$	319.7 psf	325.2 psf	316.1 psf	304.9 psf	327.8 psf	303.2 psf	124.6 psf	124.2 psf	120.7 psf



Maximum Bearing Pressure = 328 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 33in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



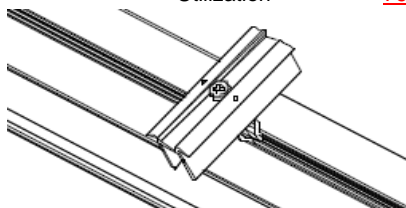
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

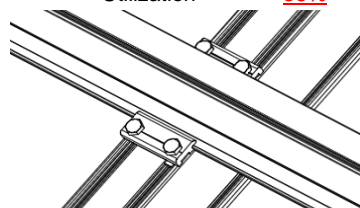
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.923 k
Allowable Uplift =	1.214 k
Utilization =	<u>76%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.320 k
Allowable Uplift =	4.357 k
Utilization =	<u>53%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.838 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>25%</u>

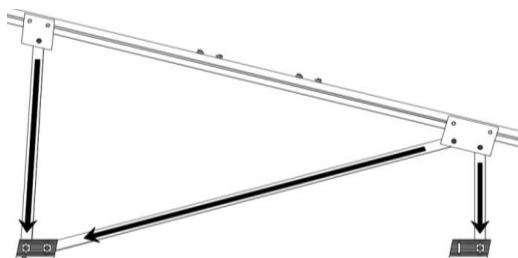
#### Rear Strut

Maximum Axial Load =	4.337 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>58%</u>

#### Diagonal Strut

Maximum Axial Load =	2.869 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

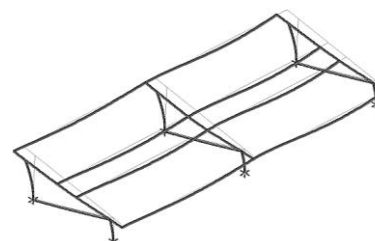
## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u>0.428 ≤ 1.076, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 84 \text{ in}$$

$$J = 0.432$$

$$232.383$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 84$$

$$J = 0.432$$

$$147.782$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-75.661	-75.661	0	0
2	M14	y	-75.661	-75.661	0	0
3	M15	y	-126.102	-126.102	0	0
4	M16	y	-126.102	-126.102	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	170.238	170.238	0	0
2	M14	y	132.407	132.407	0	0
3	M15	y	75.661	75.661	0	0
4	M16	y	75.661	75.661	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 35° 120mph 30psf 7ft 7-05.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	52.225	4	640.281	2	-5.198	12	.002	3	.171	1	.948	2
20			min	5.376	10	-1168.915	3	-137.461	1	-.012	2	0	3	-1.617	3
21		11	max	49.177	1	528.076	2	-3.604	12	.012	2	.089	4	.493	2
22			min	5.376	10	-960.478	3	-108.643	1	0	15	-.005	3	-.789	3
23		12	max	49.177	1	415.871	2	-2.011	12	.012	2	.045	4	.126	2
24			min	5.376	10	-752.04	3	-79.826	1	0	15	-.008	3	-.123	3
25		13	max	49.177	1	303.666	2	-.397	3	.012	2	.021	5	.381	3
26			min	5.376	10	-543.603	3	-51.009	1	0	15	-.049	1	-.154	2
27		14	max	49.177	1	191.461	2	1.993	3	.012	2	0	15	.723	3
28			min	4.961	15	-335.165	3	-32.531	4	0	15	-.077	1	-.346	2
29		15	max	49.177	1	79.256	2	6.626	1	.012	2	-.004	12	.903	3
30			min	-.47	5	-126.727	3	-24.611	5	0	15	-.083	1	-.452	2
31		16	max	49.177	1	81.71	3	35.443	1	.012	2	-.001	12	.92	3
32			min	-8.454	5	-32.949	2	-22.145	5	0	15	-.067	1	-.47	2
33		17	max	49.177	1	290.148	3	64.261	1	.012	2	.004	3	.776	3
34			min	-16.438	5	-145.154	2	-19.679	5	0	15	-.063	4	-.4	2
35		18	max	49.177	1	498.585	3	93.078	1	.012	2	.033	1	.469	3
36			min	-24.422	5	-257.359	2	-17.214	5	0	15	-.07	5	-.244	2
37		19	max	49.177	1	707.023	3	121.895	1	.012	2	.117	1	0	2
38			min	-32.406	5	-369.564	2	-14.748	5	0	15	-.082	5	0	3
39	M14	1	max	32.888	4	428.997	2	-9.463	12	.011	3	.197	4	0	4
40			min	2.831	12	-586.55	3	-126.899	1	-.012	2	.016	10	0	3
41		2	max	28.677	1	316.792	2	-7.869	12	.011	3	.134	4	.393	3
42			min	2.831	12	-425.191	3	-98.082	1	-.012	2	.004	10	-.29	2
43		3	max	28.677	1	204.587	2	-6.275	12	.011	3	.081	5	.661	3
44			min	2.831	12	-263.832	3	-69.265	1	-.012	2	-.013	1	-.493	2
45		4	max	28.677	1	92.382	2	-4.536	10	.011	3	.045	5	.804	3
46			min	1.039	15	-102.473	3	-55.854	4	-.012	2	-.055	1	-.608	2
47		5	max	28.677	1	58.886	3	-.133	10	.011	3	.011	5	.821	3
48			min	-6.403	5	-19.823	2	-46.131	4	-.012	2	-.076	1	-.636	2
49		6	max	28.677	1	220.245	3	17.187	1	.011	3	-.005	12	.712	3
50			min	-14.387	5	-132.028	2	-39.974	5	-.012	2	-.073	1	-.577	2
51		7	max	28.677	1	381.604	3	46.005	1	.011	3	-.005	10	.478	3
52			min	-22.371	5	-244.233	2	-37.508	5	-.012	2	-.064	4	-.431	2
53		8	max	28.677	1	542.963	3	74.822	1	.011	3	.004	2	.119	3
54			min	-30.355	5	-356.437	2	-35.042	5	-.012	2	-.081	4	-.198	2
55		9	max	28.677	1	704.322	3	103.639	1	.011	3	.068	1	.123	2
56			min	-38.339	5	-468.642	2	-32.576	5	-.012	2	-.105	5	-.366	3
57		10	max	58.611	4	580.847	2	-4.881	12	.011	3	.196	4	.531	2
58			min	2.831	12	-865.681	3	-132.457	1	-.012	2	0	3	-.977	3
59		11	max	50.627	4	468.642	2	-3.287	12	.012	2	.133	4	.123	2
60			min	2.831	12	-704.322	3	-103.639	1	-.011	3	-.005	3	-.366	3
61		12	max	42.643	4	356.437	2	-1.693	12	.012	2	.078	5	.119	3
62			min	2.831	12	-542.963	3	-74.822	1	-.011	3	-.008	3	-.198	2
63		13	max	34.659	4	244.233	2	.082	3	.012	2	.042	5	.478	3
64			min	2.831	12	-381.604	3	-56.718	4	-.011	3	-.049	1	-.431	2
65		14	max	28.677	1	132.028	2	2.473	3	.012	2	.008	5	.712	3
66			min	2.831	12	-220.245	3	-46.995	4	-.011	3	-.073	1	-.577	2
67		15	max	28.677	1	19.823	2	11.63	1	.012	2	-.003	12	.821	3
68			min	2.831	12	-58.886	3	-40.193	5	-.011	3	-.076	1	-.636	2
69		16	max	28.677	1	102.473	3	40.447	1	.012	2	0	3	.804	3
70			min	2.095	15	-92.382	2	-37.727	5	-.011	3	-.068	4	-.608	2
71		17	max	28.677	1	263.832	3	69.265	1	.012	2	.006	3	.661	3
72			min	-4.81	5	-204.587	2	-35.262	5	-.011	3	-.086	4	-.493	2
73		18	max	28.677	1	425.191	3	98.082	1	.012	2	.052	1	.393	3
74			min	-12.794	5	-316.792	2	-32.796	5	-.011	3	-.109	5	-.29	2
75		19	max	28.677	1	586.55	3	126.899	1	.012	2	.14	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-20.778	5	-428.997	2	-30.33	5	-.011	3	-.134	5	0	3
77	M15	1	max	64.056	5	640.919	2	-9.267	12	.012	2	.244	4	0	2
78			min	-29.467	1	-348.553	3	-126.94	1	-.01	3	.016	12	0	3
79		2	max	56.072	5	465.943	2	-7.673	12	.012	2	.171	4	.236	3
80			min	-29.467	1	-257.811	3	-98.123	1	-.01	3	.004	10	-.43	2
81		3	max	48.088	5	290.967	2	-6.079	12	.012	2	.108	5	.401	3
82			min	-29.467	1	-167.069	3	-78.825	4	-.01	3	-.013	1	-.725	2
83		4	max	40.104	5	115.991	2	-4.485	12	.012	2	.062	5	.496	3
84			min	-29.467	1	-76.327	3	-69.102	4	-.01	3	-.055	1	-.883	2
85		5	max	32.12	5	14.414	3	-.233	10	.012	2	.017	5	.52	3
86			min	-29.467	1	-58.985	2	-59.379	4	-.01	3	-.076	1	-.905	2
87		6	max	24.136	5	105.156	3	17.147	1	.012	2	-.005	12	.473	3
88			min	-29.467	1	-233.961	2	-53.182	5	-.01	3	-.073	1	-.791	2
89		7	max	16.152	5	195.898	3	45.964	1	.012	2	-.005	12	.356	3
90			min	-29.467	1	-408.937	2	-50.716	5	-.01	3	-.079	4	-.541	2
91		8	max	8.168	5	286.64	3	74.781	1	.012	2	.004	2	.169	3
92			min	-29.467	1	-583.913	2	-48.25	5	-.01	3	-.106	4	-.155	2
93		9	max	.226	15	377.381	3	103.599	1	.012	2	.067	1	.367	2
94			min	-29.467	1	-758.889	2	-45.784	5	-.01	3	-.14	5	-.09	3
95		10	max	-3.161	10	933.865	2	-5.077	12	.012	2	.242	4	1.025	2
96			min	-29.467	1	-468.123	3	-132.416	1	-.01	3	0	3	-.418	3
97		11	max	-3.161	10	758.889	2	-3.483	12	.01	3	.169	4	.367	2
98			min	-29.467	1	-377.381	3	-103.599	1	-.012	2	-.004	3	-.09	3
99		12	max	-3.161	10	583.913	2	-1.89	12	.01	3	.103	5	.169	3
100			min	-29.467	1	-286.64	3	-79.717	4	-.012	2	-.007	3	-.155	2
101		13	max	-3.161	10	408.937	2	-.231	3	.01	3	.057	5	.356	3
102			min	-29.932	4	-195.898	3	-69.995	4	-.012	2	-.049	1	-.541	2
103		14	max	-3.161	10	233.961	2	2.159	3	.01	3	.012	5	.473	3
104			min	-37.916	4	-105.156	3	-60.272	4	-.012	2	-.073	1	-.791	2
105		15	max	-3.161	10	58.985	2	11.671	1	.01	3	-.003	12	.52	3
106			min	-45.9	4	-14.414	3	-53.408	5	-.012	2	-.076	1	-.905	2
107		16	max	-3.161	10	76.327	3	40.488	1	.01	3	0	12	.496	3
108			min	-53.884	4	-115.991	2	-50.942	5	-.012	2	-.085	4	-.883	2
109		17	max	-3.161	10	167.069	3	69.305	1	.01	3	.006	3	.401	3
110			min	-61.869	4	-290.967	2	-48.476	5	-.012	2	-.113	4	-.725	2
111		18	max	-3.161	10	257.811	3	98.123	1	.01	3	.053	1	.236	3
112			min	-69.853	4	-465.943	2	-46.01	5	-.012	2	-.146	5	-.43	2
113		19	max	-3.161	10	348.553	3	126.94	1	.01	3	.14	1	0	2
114			min	-77.837	4	-640.919	2	-43.544	5	-.012	2	-.181	5	0	5
115	M16	1	max	61.9	5	584.473	2	-8.498	12	.007	2	.187	4	0	2
116			min	-53.678	1	-296.753	3	-122.322	1	-.011	3	.012	12	0	3
117		2	max	53.916	5	409.497	2	-6.905	12	.007	2	.127	4	.196	3
118			min	-53.678	1	-206.011	3	-93.505	1	-.011	3	.002	10	-.387	2
119		3	max	45.932	5	234.521	2	-5.311	12	.007	2	.081	5	.32	3
120			min	-53.678	1	-115.269	3	-64.688	1	-.011	3	-.027	1	-.637	2
121		4	max	37.948	5	59.545	2	-3.717	12	.007	2	.047	5	.375	3
122			min	-53.678	1	-24.528	3	-52.193	4	-.011	3	-.066	1	-.751	2
123		5	max	29.964	5	66.214	3	.175	10	.007	2	.015	5	.359	3
124			min	-53.678	1	-115.431	2	-42.47	4	-.011	3	-.083	1	-.73	2
125		6	max	21.98	5	156.956	3	21.764	1	.007	2	-.005	12	.272	3
126			min	-53.678	1	-290.407	2	-37.614	5	-.011	3	-.077	1	-.572	2
127		7	max	13.996	5	247.698	3	50.582	1	.007	2	-.005	12	.114	3
128			min	-53.678	1	-465.383	2	-35.149	5	-.011	3	-.057	4	-.278	2
129		8	max	6.012	5	338.439	3	79.399	1	.007	2	.005	2	.152	2
130			min	-53.678	1	-640.359	2	-32.683	5	-.011	3	-.071	4	-.113	3
131		9	max	-1.212	15	429.181	3	108.216	1	.007	2	.075	1	.718	2
132			min	-53.678	1	-815.335	2	-30.217	5	-.011	3	-.094	5	-.412	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-5.635	12	990.311	2	-5.845	12	.007	2	.188	4	1.42	2
134		min	-53.678	1	-519.923	3	-137.034	1	-.011	3	.003	12	-.781	3
135	11	max	-4.996	15	815.335	2	-4.251	12	.011	3	.125	4	.718	2
136		min	-53.678	1	-429.181	3	-108.216	1	-.007	2	-.002	3	-.412	3
137	12	max	-5.635	12	640.359	2	-2.658	12	.011	3	.07	4	.152	2
138		min	-53.678	1	-338.439	3	-79.399	1	-.007	2	-.006	3	-.113	3
139	13	max	-5.635	12	465.383	2	-1.064	12	.011	3	.035	5	.114	3
140		min	-53.678	1	-247.698	3	-56.387	4	-.007	2	-.049	1	-.278	2
141	14	max	-5.635	12	290.407	2	.958	3	.011	3	.002	5	.272	3
142		min	-53.678	1	-156.956	3	-46.664	4	-.007	2	-.077	1	-.572	2
143	15	max	-5.635	12	115.431	2	7.053	1	.011	3	-.004	12	.359	3
144		min	-53.925	4	-66.214	3	-38.68	5	-.007	2	-.083	1	-.73	2
145	16	max	-5.635	12	24.528	3	35.87	1	.011	3	-.002	12	.375	3
146		min	-61.91	4	-59.545	2	-36.215	5	-.007	2	-.075	4	-.751	2
147	17	max	-5.635	12	115.269	3	64.688	1	.011	3	.003	3	.32	3
148		min	-69.894	4	-234.521	2	-33.749	5	-.007	2	-.092	4	-.637	2
149	18	max	-5.635	12	206.011	3	93.505	1	.011	3	.034	1	.196	3
150		min	-77.878	4	-409.497	2	-31.283	5	-.007	2	-.111	5	-.387	2
151	19	max	-5.635	12	296.753	3	122.322	1	.011	3	.118	1	0	2
152		min	-85.862	4	-584.473	2	-28.817	5	-.007	2	-.134	5	0	5
153	M2	1	max	947.831	2	2.044	4	.165	1	0	0	3	0	1
154		min	-1382.186	3	.491	15	-14.223	4	0	4	0	2	0	1
155	2	max	948.352	2	1.925	4	.165	1	0	3	0	1	0	15
156		min	-1381.795	3	.463	15	-14.682	4	0	4	-.005	4	0	4
157	3	max	948.873	2	1.806	4	.165	1	0	3	0	1	0	15
158		min	-1381.405	3	.435	15	-15.14	4	0	4	-.01	4	-.001	4
159	4	max	949.393	2	1.688	4	.165	1	0	3	0	1	0	15
160		min	-1381.014	3	.407	15	-15.598	4	0	4	-.016	4	-.002	4
161	5	max	949.914	2	1.569	4	.165	1	0	3	0	1	0	15
162		min	-1380.624	3	.38	15	-16.057	4	0	4	-.022	4	-.003	4
163	6	max	950.435	2	1.45	4	.165	1	0	3	0	1	0	15
164		min	-1380.233	3	.352	15	-16.515	4	0	4	-.027	4	-.003	4
165	7	max	950.956	2	1.331	4	.165	1	0	3	0	1	0	15
166		min	-1379.843	3	.324	15	-16.973	4	0	4	-.033	4	-.004	4
167	8	max	951.476	2	1.212	4	.165	1	0	3	0	1	0	15
168		min	-1379.452	3	.289	12	-17.432	4	0	4	-.039	4	-.004	4
169	9	max	951.997	2	1.093	4	.165	1	0	3	0	1	-.001	15
170		min	-1379.062	3	.242	12	-17.89	4	0	4	-.046	4	-.004	4
171	10	max	952.518	2	.974	4	.165	1	0	3	0	1	-.001	15
172		min	-1378.671	3	.196	12	-18.348	4	0	4	-.052	4	-.005	4
173	11	max	953.038	2	.856	4	.165	1	0	3	0	1	-.001	15
174		min	-1378.28	3	.15	12	-18.807	4	0	4	-.059	4	-.005	4
175	12	max	953.559	2	.759	2	.165	1	0	3	0	1	-.001	15
176		min	-1377.89	3	.104	12	-19.265	4	0	4	-.066	4	-.005	4
177	13	max	954.08	2	.666	2	.165	1	0	3	0	1	-.001	15
178		min	-1377.499	3	.057	12	-19.723	4	0	4	-.073	4	-.006	4
179	14	max	954.6	2	.574	2	.165	1	0	3	0	1	-.001	15
180		min	-1377.109	3	-.001	3	-20.182	4	0	4	-.08	4	-.006	4
181	15	max	955.121	2	.481	2	.165	1	0	3	0	1	-.001	12
182		min	-1376.718	3	-.071	3	-20.64	4	0	4	-.087	4	-.006	4
183	16	max	955.642	2	.389	2	.165	1	0	3	0	1	-.001	12
184		min	-1376.328	3	-.14	3	-21.098	4	0	4	-.094	4	-.006	4
185	17	max	956.163	2	.296	2	.165	1	0	3	0	1	-.001	12
186		min	-1375.937	3	-.21	3	-21.557	4	0	4	-.102	4	-.006	4
187	18	max	956.683	2	.203	2	.165	1	0	3	0	1	-.001	12
188		min	-1375.547	3	-.279	3	-22.015	4	0	4	-.11	4	-.006	4
189	19	max	957.204	2	.111	2	.165	1	0	3	.001	1	-.001	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1375.156	3	-.348	3	-22.474	4	0	4	-.118	4	-.006	4
191	M3	1	max	856.557	2	7.684	4	4.939	4	0	3	0	.006	4
192		min	-951.878	3	1.815	15	.018	12	0	4	-.021	4	.001	12
193		2	max	856.386	2	6.923	4	5.474	4	0	3	0	.004	2
194		min	-952.006	3	1.637	15	.018	12	0	4	-.019	4	0	3
195		3	max	856.216	2	6.162	4	6.009	4	0	3	0	.001	2
196		min	-952.134	3	1.458	15	.018	12	0	4	-.017	4	-.001	3
197		4	max	856.046	2	5.401	4	6.543	4	0	3	0	0	15
198		min	-952.262	3	1.279	15	.018	12	0	4	-.014	5	-.003	3
199		5	max	855.875	2	4.64	4	7.078	4	0	3	0	0	15
200		min	-952.389	3	1.1	15	.018	12	0	4	-.012	5	-.004	6
201		6	max	855.705	2	3.879	4	7.613	4	0	3	0	1	15
202		min	-952.517	3	.921	15	.018	12	0	4	-.009	5	-.006	6
203		7	max	855.535	2	3.118	4	8.147	4	0	3	0	1	15
204		min	-952.645	3	.742	15	.018	12	0	4	-.005	5	-.007	6
205		8	max	855.364	2	2.357	4	8.682	4	0	3	0	1	15
206		min	-952.773	3	.563	15	.018	12	0	4	-.002	5	-.008	6
207		9	max	855.194	2	1.596	4	9.217	4	0	3	.002	4	15
208		min	-952.9	3	.384	15	.018	12	0	4	0	12	-.009	6
209		10	max	855.024	2	.835	4	9.751	4	0	3	.006	4	15
210		min	-953.028	3	.168	12	.018	12	0	4	0	12	-.01	6
211		11	max	854.853	2	.216	2	10.286	4	0	3	.01	4	15
212		min	-953.156	3	-.208	3	.018	12	0	4	0	12	-.01	6
213		12	max	854.683	2	-.152	15	10.821	4	0	3	.015	4	15
214		min	-953.284	3	-.687	6	.018	12	0	4	0	12	-.01	6
215		13	max	854.513	2	-.331	15	11.355	4	0	3	.019	4	15
216		min	-953.412	3	-1.448	6	.018	12	0	4	0	12	-.009	6
217		14	max	854.342	2	-.51	15	11.89	4	0	3	.024	4	15
218		min	-953.539	3	-2.209	6	.018	12	0	4	0	12	-.009	6
219		15	max	854.172	2	-.689	15	12.425	4	0	3	.029	4	15
220		min	-953.667	3	-2.97	6	.018	12	0	4	0	12	-.007	6
221		16	max	854.002	2	-.868	15	12.96	4	0	3	.034	4	15
222		min	-953.795	3	-3.731	6	.018	12	0	4	0	12	-.006	6
223		17	max	853.831	2	-1.047	15	13.494	4	0	3	.04	4	15
224		min	-953.923	3	-4.492	6	.018	12	0	4	0	12	-.004	6
225		18	max	853.661	2	-1.225	15	14.029	4	0	3	.046	4	15
226		min	-954.05	3	-5.253	6	.018	12	0	4	0	12	-.002	6
227		19	max	853.491	2	-1.404	15	14.564	4	0	3	.052	4	1
228		min	-954.178	3	-6.014	6	.018	12	0	4	0	12	0	1
229	M4	1	max	711.046	1	0	1	-.581	12	0	1	.049	4	1
230		min	-78.211	5	0	1	-220.467	4	0	1	0	12	0	1
231		2	max	711.216	1	0	1	-.581	12	0	1	.024	4	1
232		min	-78.132	5	0	1	-220.615	4	0	1	0	10	0	1
233		3	max	711.387	1	0	1	-.581	12	0	1	0	1	1
234		min	-78.052	5	0	1	-220.762	4	0	1	-.002	4	0	1
235		4	max	711.557	1	0	1	-.581	12	0	1	0	12	1
236		min	-77.973	5	0	1	-220.91	4	0	1	-.027	4	0	1
237		5	max	711.727	1	0	1	-.581	12	0	1	0	12	1
238		min	-77.893	5	0	1	-221.058	4	0	1	-.052	4	0	1
239		6	max	711.898	1	0	1	-.581	12	0	1	0	12	1
240		min	-77.814	5	0	1	-221.205	4	0	1	-.078	4	0	1
241		7	max	712.068	1	0	1	-.581	12	0	1	0	12	1
242		min	-77.734	5	0	1	-221.353	4	0	1	-.103	4	0	1
243		8	max	712.238	1	0	1	-.581	12	0	1	0	12	1
244		min	-77.655	5	0	1	-221.501	4	0	1	-.129	4	0	1
245		9	max	712.409	1	0	1	-.581	12	0	1	0	12	1
246		min	-77.575	5	0	1	-221.648	4	0	1	-.154	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	712.579	1	0	1	-.581	12	0	1	0	12	0	1
248		min	-77.496	5	0	1	-221.796	4	0	1	-.18	4	0	1
249	11	max	712.749	1	0	1	-.581	12	0	1	0	12	0	1
250		min	-77.416	5	0	1	-221.944	4	0	1	-.205	4	0	1
251	12	max	712.92	1	0	1	-.581	12	0	1	0	12	0	1
252		min	-77.337	5	0	1	-222.091	4	0	1	-.231	4	0	1
253	13	max	713.09	1	0	1	-.581	12	0	1	0	12	0	1
254		min	-77.257	5	0	1	-222.239	4	0	1	-.256	4	0	1
255	14	max	713.261	1	0	1	-.581	12	0	1	0	12	0	1
256		min	-77.178	5	0	1	-222.386	4	0	1	-.282	4	0	1
257	15	max	713.431	1	0	1	-.581	12	0	1	0	12	0	1
258		min	-77.098	5	0	1	-222.534	4	0	1	-.307	4	0	1
259	16	max	713.601	1	0	1	-.581	12	0	1	0	12	0	1
260		min	-77.019	5	0	1	-222.682	4	0	1	-.333	4	0	1
261	17	max	713.772	1	0	1	-.581	12	0	1	0	12	0	1
262		min	-76.939	5	0	1	-222.829	4	0	1	-.358	4	0	1
263	18	max	713.942	1	0	1	-.581	12	0	1	0	12	0	1
264		min	-76.86	5	0	1	-222.977	4	0	1	-.384	4	0	1
265	19	max	714.112	1	0	1	-.581	12	0	1	-.001	12	0	1
266		min	-76.78	5	0	1	-223.125	4	0	1	-.41	4	0	1
267	M6	1	max	2898.884	2	2.228	2	0	1	0	0	4	0	1
268		min	-4337.392	3	.253	12	-14.372	4	0	4	0	1	0	1
269	2	max	2899.405	2	2.135	2	0	1	0	1	0	1	0	12
270		min	-4337.002	3	.207	12	-14.831	4	0	4	-.005	4	0	2
271	3	max	2899.926	2	2.042	2	0	1	0	1	0	1	0	12
272		min	-4336.611	3	.161	12	-15.289	4	0	4	-.011	4	-.002	2
273	4	max	2900.446	2	1.95	2	0	1	0	1	0	1	0	12
274		min	-4336.221	3	.114	12	-15.747	4	0	4	-.016	4	-.002	2
275	5	max	2900.967	2	1.857	2	0	1	0	1	0	1	0	12
276		min	-4335.83	3	.049	3	-16.206	4	0	4	-.022	4	-.003	2
277	6	max	2901.488	2	1.765	2	0	1	0	1	0	1	0	12
278		min	-4335.44	3	-.021	3	-16.664	4	0	4	-.028	4	-.004	2
279	7	max	2902.008	2	1.672	2	0	1	0	1	0	1	0	12
280		min	-4335.049	3	-.09	3	-17.123	4	0	4	-.034	4	-.004	2
281	8	max	2902.529	2	1.579	2	0	1	0	1	0	1	0	3
282		min	-4334.659	3	-.16	3	-17.581	4	0	4	-.04	4	-.005	2
283	9	max	2903.05	2	1.487	2	0	1	0	1	0	1	0	3
284		min	-4334.268	3	-.229	3	-18.039	4	0	4	-.046	4	-.005	2
285	10	max	2903.571	2	1.394	2	0	1	0	1	0	1	0	3
286		min	-4333.878	3	-.299	3	-18.498	4	0	4	-.053	4	-.006	2
287	11	max	2904.091	2	1.302	2	0	1	0	1	0	1	0	3
288		min	-4333.487	3	-.368	3	-18.956	4	0	4	-.059	4	-.006	2
289	12	max	2904.612	2	1.209	2	0	1	0	1	0	1	0	3
290		min	-4333.097	3	-.438	3	-19.414	4	0	4	-.066	4	-.007	2
291	13	max	2905.133	2	1.116	2	0	1	0	1	0	1	0	3
292		min	-4332.706	3	-.507	3	-19.873	4	0	4	-.073	4	-.007	2
293	14	max	2905.653	2	1.024	2	0	1	0	1	0	1	0	3
294		min	-4332.316	3	-.576	3	-20.331	4	0	4	-.08	4	-.008	2
295	15	max	2906.174	2	.931	2	0	1	0	1	0	1	0	3
296		min	-4331.925	3	-.646	3	-20.789	4	0	4	-.088	4	-.008	2
297	16	max	2906.695	2	.838	2	0	1	0	1	0	1	.001	3
298		min	-4331.535	3	-.715	3	-21.248	4	0	4	-.095	4	-.008	2
299	17	max	2907.215	2	.746	2	0	1	0	1	0	1	.001	3
300		min	-4331.144	3	-.785	3	-21.706	4	0	4	-.103	4	-.008	2
301	18	max	2907.736	2	.653	2	0	1	0	1	0	1	.002	3
302		min	-4330.754	3	-.854	3	-22.164	4	0	4	-.111	4	-.009	2
303	19	max	2908.257	2	.561	2	0	1	0	1	0	1	.002	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4330.363	3	-.924	3	-22.623	4	0	4	-.119	4	-.009	2
305	M7	1	max	2810.836	2	7.681	6	4.641	4	0	1	0	1	.009	2
306			min	-2866.24	3	1.804	15	0	1	0	4	-.022	4	-.002	3
307		2	max	2810.666	2	6.92	6	5.175	4	0	1	0	1	.006	2
308			min	-2866.368	3	1.626	15	0	1	0	4	-.02	4	-.003	3
309		3	max	2810.496	2	6.159	6	5.71	4	0	1	0	1	.004	2
310			min	-2866.495	3	1.447	15	0	1	0	4	-.017	4	-.005	3
311		4	max	2810.325	2	5.398	6	6.245	4	0	1	0	1	.002	2
312			min	-2866.623	3	1.268	15	0	1	0	4	-.015	4	-.006	3
313		5	max	2810.155	2	4.637	6	6.78	4	0	1	0	1	0	2
314			min	-2866.751	3	1.089	15	0	1	0	4	-.012	4	-.007	3
315		6	max	2809.985	2	3.876	6	7.314	4	0	1	0	1	-.001	15
316			min	-2866.879	3	.91	15	0	1	0	4	-.009	4	-.007	3
317		7	max	2809.814	2	3.115	6	7.849	4	0	1	0	1	-.002	15
318			min	-2867.006	3	.731	15	0	1	0	4	-.006	5	-.008	3
319		8	max	2809.644	2	2.381	2	8.384	4	0	1	0	1	-.002	15
320			min	-2867.134	3	.453	12	0	1	0	4	-.003	5	-.008	4
321		9	max	2809.474	2	1.788	2	8.918	4	0	1	0	4	-.002	15
322			min	-2867.262	3	.156	12	0	1	0	4	0	1	-.009	4
323		10	max	2809.303	2	1.195	2	9.453	4	0	1	.005	4	-.002	15
324			min	-2867.39	3	-.256	3	0	1	0	4	0	1	-.01	4
325		11	max	2809.133	2	.602	2	9.988	4	0	1	.009	4	-.002	15
326			min	-2867.517	3	-.701	3	0	1	0	4	0	1	-.01	4
327		12	max	2808.963	2	.009	2	10.522	4	0	1	.013	4	-.002	15
328			min	-2867.645	3	-1.145	3	0	1	0	4	0	1	-.01	4
329		13	max	2808.792	2	-.342	15	11.057	4	0	1	.018	4	-.002	15
330			min	-2867.773	3	-1.59	3	0	1	0	4	0	1	-.009	4
331		14	max	2808.622	2	-.521	15	11.592	4	0	1	.022	4	-.002	15
332			min	-2867.901	3	-2.211	4	0	1	0	4	0	1	-.009	4
333		15	max	2808.451	2	-.7	15	12.126	4	0	1	.027	4	-.002	15
334			min	-2868.028	3	-2.972	4	0	1	0	4	0	1	-.007	4
335		16	max	2808.281	2	-.879	15	12.661	4	0	1	.032	4	-.001	15
336			min	-2868.156	3	-3.733	4	0	1	0	4	0	1	-.006	4
337		17	max	2808.111	2	-1.058	15	13.196	4	0	1	.038	4	-.001	15
338			min	-2868.284	3	-4.494	4	0	1	0	4	0	1	-.004	4
339		18	max	2807.94	2	-1.237	15	13.731	4	0	1	.043	4	0	15
340			min	-2868.412	3	-5.255	4	0	1	0	4	0	1	-.002	4
341		19	max	2807.77	2	-1.415	15	14.265	4	0	1	.049	4	0	1
342			min	-2868.54	3	-6.016	4	0	1	0	4	0	1	0	1
343	M8	1	max	1834.662	2	0	1	0	1	0	1	.047	4	0	1
344			min	68.106	15	0	1	-212.993	4	0	1	0	1	0	1
345		2	max	1834.832	2	0	1	0	1	0	1	.022	4	0	1
346			min	68.158	15	0	1	-213.141	4	0	1	0	1	0	1
347		3	max	1835.002	2	0	1	0	1	0	1	0	1	0	1
348			min	68.209	15	0	1	-213.288	4	0	1	-.002	4	0	1
349		4	max	1835.173	2	0	1	0	1	0	1	0	1	0	1
350			min	68.261	15	0	1	-213.436	4	0	1	-.027	4	0	1
351		5	max	1835.343	2	0	1	0	1	0	1	0	1	0	1
352			min	68.312	15	0	1	-213.584	4	0	1	-.051	4	0	1
353		6	max	1835.513	2	0	1	0	1	0	1	0	1	0	1
354			min	68.363	15	0	1	-213.731	4	0	1	-.076	4	0	1
355		7	max	1835.684	2	0	1	0	1	0	1	0	1	0	1
356			min	68.415	15	0	1	-213.879	4	0	1	-.1	4	0	1
357		8	max	1835.854	2	0	1	0	1	0	1	0	1	0	1
358			min	68.466	15	0	1	-214.027	4	0	1	-.125	4	0	1
359		9	max	1836.024	2	0	1	0	1	0	1	0	1	0	1
360			min	68.518	15	0	1	-214.174	4	0	1	-.15	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	1836.195	2	0	1	0	1	0	1	0	1	0	1
362			min	68.569	15	0	1	-214.322	4	0	1	-.174	4	0	1
363		11	max	1836.365	2	0	1	0	1	0	1	0	1	0	1
364			min	68.62	15	0	1	-214.47	4	0	1	-.199	4	0	1
365		12	max	1836.535	2	0	1	0	1	0	1	0	1	0	1
366			min	68.672	15	0	1	-214.617	4	0	1	-.223	4	0	1
367		13	max	1836.706	2	0	1	0	1	0	1	0	1	0	1
368			min	68.723	15	0	1	-214.765	4	0	1	-.248	4	0	1
369		14	max	1836.876	2	0	1	0	1	0	1	0	1	0	1
370			min	68.774	15	0	1	-214.912	4	0	1	-.273	4	0	1
371		15	max	1837.046	2	0	1	0	1	0	1	0	1	0	1
372			min	68.826	15	0	1	-215.06	4	0	1	-.297	4	0	1
373		16	max	1837.217	2	0	1	0	1	0	1	0	1	0	1
374			min	68.877	15	0	1	-215.208	4	0	1	-.322	4	0	1
375		17	max	1837.387	2	0	1	0	1	0	1	0	1	0	1
376			min	68.929	15	0	1	-215.355	4	0	1	-.347	4	0	1
377		18	max	1837.557	2	0	1	0	1	0	1	0	1	0	1
378			min	68.98	15	0	1	-215.503	4	0	1	-.372	4	0	1
379		19	max	1837.728	2	0	1	0	1	0	1	0	1	0	1
380			min	69.031	15	0	1	-215.651	4	0	1	-.396	4	0	1
381	M10	1	max	947.831	2	1.996	6	-.019	10	0	1	0	4	0	1
382			min	-1382.186	3	.458	15	-14.339	4	0	5	0	3	0	1
383		2	max	948.352	2	1.877	6	-.019	10	0	1	0	10	0	15
384			min	-1381.795	3	.431	15	-14.797	4	0	5	-.005	4	0	6
385		3	max	948.873	2	1.758	6	-.019	10	0	1	0	10	0	15
386			min	-1381.405	3	.403	15	-15.256	4	0	5	-.011	4	-.001	6
387		4	max	949.393	2	1.639	6	-.019	10	0	1	0	10	0	15
388			min	-1381.014	3	.375	15	-15.714	4	0	5	-.016	4	-.002	6
389		5	max	949.914	2	1.52	6	-.019	10	0	1	0	10	0	15
390			min	-1380.624	3	.347	15	-16.172	4	0	5	-.022	4	-.003	6
391		6	max	950.435	2	1.402	6	-.019	10	0	1	0	10	0	15
392			min	-1380.233	3	.319	15	-16.631	4	0	5	-.028	4	-.003	6
393		7	max	950.956	2	1.283	6	-.019	10	0	1	0	10	0	15
394			min	-1379.843	3	.291	15	-17.089	4	0	5	-.034	4	-.004	6
395		8	max	951.476	2	1.164	6	-.019	10	0	1	0	10	0	15
396			min	-1379.452	3	.263	15	-17.547	4	0	5	-.04	4	-.004	6
397		9	max	951.997	2	1.045	6	-.019	10	0	1	0	10	0	15
398			min	-1379.062	3	.235	15	-18.006	4	0	5	-.046	4	-.004	6
399		10	max	952.518	2	.944	2	-.019	10	0	1	0	10	-.001	15
400			min	-1378.671	3	.196	12	-18.464	4	0	5	-.053	4	-.005	6
401		11	max	953.038	2	.852	2	-.019	10	0	1	0	10	-.001	15
402			min	-1378.28	3	.15	12	-18.922	4	0	5	-.059	4	-.005	6
403		12	max	953.559	2	.759	2	-.019	10	0	1	0	10	-.001	15
404			min	-1377.89	3	.104	12	-19.381	4	0	5	-.066	4	-.005	6
405		13	max	954.08	2	.666	2	-.019	10	0	1	0	10	-.001	15
406			min	-1377.499	3	.057	12	-19.839	4	0	5	-.073	4	-.005	6
407		14	max	954.6	2	.574	2	-.019	10	0	1	0	10	-.001	15
408			min	-1377.109	3	-.001	3	-20.298	4	0	5	-.08	4	-.006	6
409		15	max	955.121	2	.481	2	-.019	10	0	1	0	10	-.001	15
410			min	-1376.718	3	-.071	3	-20.756	4	0	5	-.088	4	-.006	6
411		16	max	955.642	2	.389	2	-.019	10	0	1	0	10	-.001	15
412			min	-1376.328	3	-.14	3	-21.214	4	0	5	-.095	4	-.006	6
413		17	max	956.163	2	.296	2	-.019	10	0	1	0	10	-.001	15
414			min	-1375.937	3	-.21	3	-21.673	4	0	5	-.103	4	-.006	6
415		18	max	956.683	2	.203	2	-.019	10	0	1	0	10	-.001	12
416			min	-1375.547	3	-.279	3	-22.131	4	0	5	-.111	4	-.006	2
417		19	max	957.204	2	.111	2	-.019	10	0	1	0	10	-.001	12



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1375.156	3	-.348	3	-22.589	4	0	5	-.118	4	-.006	2
419	M11	1	max	856.557	2	7.643	6	4.822	4	0	1	0	10	.006	2
420			min	-951.878	3	1.787	15	-.169	1	0	4	-.022	4	.001	12
421		2	max	856.386	2	6.882	6	5.356	4	0	1	0	10	.004	2
422			min	-952.006	3	1.608	15	-.169	1	0	4	-.019	4	0	3
423		3	max	856.216	2	6.121	6	5.891	4	0	1	0	10	.001	2
424			min	-952.134	3	1.43	15	-.169	1	0	4	-.017	4	-.001	3
425		4	max	856.046	2	5.36	6	6.426	4	0	1	0	12	0	2
426			min	-952.262	3	1.251	15	-.169	1	0	4	-.015	4	-.003	3
427		5	max	855.875	2	4.599	6	6.96	4	0	1	0	12	-.001	15
428			min	-952.389	3	1.072	15	-.169	1	0	4	-.012	4	-.004	4
429		6	max	855.705	2	3.838	6	7.495	4	0	1	0	12	-.001	15
430			min	-952.517	3	.893	15	-.169	1	0	4	-.009	4	-.006	4
431		7	max	855.535	2	3.077	6	8.03	4	0	1	0	12	-.002	15
432			min	-952.645	3	.714	15	-.169	1	0	4	-.006	4	-.007	4
433		8	max	855.364	2	2.316	6	8.564	4	0	1	0	12	-.002	15
434			min	-952.773	3	.535	15	-.169	1	0	4	-.002	4	-.009	4
435		9	max	855.194	2	1.555	6	9.099	4	0	1	.002	5	-.002	15
436			min	-.952.9	3	.356	15	-.169	1	0	4	0	1	-.009	4
437		10	max	855.024	2	.809	2	9.634	4	0	1	.006	5	-.002	15
438			min	-953.028	3	.168	12	-.169	1	0	4	0	1	-.01	4
439		11	max	854.853	2	.216	2	10.168	4	0	1	.01	5	-.002	15
440			min	-953.156	3	-.208	3	-.169	1	0	4	0	1	-.01	4
441		12	max	854.683	2	-.18	15	10.703	4	0	1	.014	5	-.002	15
442			min	-953.284	3	-.729	4	-.169	1	0	4	0	1	-.01	4
443		13	max	854.513	2	-.359	15	11.238	4	0	1	.019	5	-.002	15
444			min	-953.412	3	-1.49	4	-.169	1	0	4	-.001	1	-.009	4
445		14	max	854.342	2	-.538	15	11.772	4	0	1	.023	5	-.002	15
446			min	-953.539	3	-2.251	4	-.169	1	0	4	-.001	1	-.009	4
447		15	max	854.172	2	-.717	15	12.307	4	0	1	.028	5	-.002	15
448			min	-953.667	3	-3.012	4	-.169	1	0	4	-.001	1	-.008	4
449		16	max	854.002	2	-.896	15	12.842	4	0	1	.034	5	-.001	15
450			min	-953.795	3	-3.773	4	-.169	1	0	4	-.001	1	-.006	4
451		17	max	853.831	2	-1.075	15	13.377	4	0	1	.039	4	-.001	15
452			min	-953.923	3	-4.534	4	-.169	1	0	4	-.001	1	-.004	4
453		18	max	853.661	2	-1.254	15	13.911	4	0	1	.045	4	0	15
454			min	-954.05	3	-5.295	4	-.169	1	0	4	-.001	1	-.002	4
455		19	max	853.491	2	-1.432	15	14.446	4	0	1	.051	4	0	1
456			min	-954.178	3	-6.056	4	-.169	1	0	4	-.001	1	0	1
457	M12	1	max	711.046	1	0	1	5.707	1	0	1	.048	4	0	1
458			min	59.26	12	0	1	-216.494	4	0	1	-.001	1	0	1
459		2	max	711.216	1	0	1	5.707	1	0	1	.023	5	0	1
460			min	59.345	12	0	1	-216.641	4	0	1	0	1	0	1
461		3	max	711.387	1	0	1	5.707	1	0	1	0	10	0	1
462			min	59.43	12	0	1	-216.789	4	0	1	-.002	4	0	1
463		4	max	711.557	1	0	1	5.707	1	0	1	0	1	0	1
464			min	59.515	12	0	1	-216.937	4	0	1	-.027	4	0	1
465		5	max	711.727	1	0	1	5.707	1	0	1	.001	1	0	1
466			min	59.6	12	0	1	-217.084	4	0	1	-.052	4	0	1
467		6	max	711.898	1	0	1	5.707	1	0	1	.002	1	0	1
468			min	59.685	12	0	1	-217.232	4	0	1	-.077	4	0	1
469		7	max	712.068	1	0	1	5.707	1	0	1	.003	1	0	1
470			min	59.771	12	0	1	-217.38	4	0	1	-.101	4	0	1
471		8	max	712.238	1	0	1	5.707	1	0	1	.003	1	0	1
472			min	59.856	12	0	1	-217.527	4	0	1	-.126	4	0	1
473		9	max	712.409	1	0	1	5.707	1	0	1	.004	1	0	1
474			min	59.941	12	0	1	-217.675	4	0	1	-.151	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	712.579	1	0	1	5.707	1	0	1	.005	1	0	1
476			min	60.026	12	0	1	-217.822	4	0	1	-.176	4	0	1
477		11	max	712.749	1	0	1	5.707	1	0	1	.005	1	0	1
478			min	60.111	12	0	1	-217.97	4	0	1	-.201	4	0	1
479		12	max	712.92	1	0	1	5.707	1	0	1	.006	1	0	1
480			min	60.197	12	0	1	-218.118	4	0	1	-.227	4	0	1
481		13	max	713.09	1	0	1	5.707	1	0	1	.006	1	0	1
482			min	60.282	12	0	1	-218.265	4	0	1	-.252	4	0	1
483		14	max	713.261	1	0	1	5.707	1	0	1	.007	1	0	1
484			min	60.367	12	0	1	-218.413	4	0	1	-.277	4	0	1
485		15	max	713.431	1	0	1	5.707	1	0	1	.008	1	0	1
486			min	60.452	12	0	1	-218.561	4	0	1	-.302	4	0	1
487		16	max	713.601	1	0	1	5.707	1	0	1	.008	1	0	1
488			min	60.537	12	0	1	-218.708	4	0	1	-.327	4	0	1
489		17	max	713.772	1	0	1	5.707	1	0	1	.009	1	0	1
490			min	60.622	12	0	1	-218.856	4	0	1	-.352	4	0	1
491		18	max	713.942	1	0	1	5.707	1	0	1	.01	1	0	1
492			min	60.708	12	0	1	-219.004	4	0	1	-.377	4	0	1
493		19	max	714.112	1	0	1	5.707	1	0	1	.01	1	0	1
494			min	60.793	12	0	1	-219.151	4	0	1	-.402	4	0	1
495	M1	1	max	121.899	1	706.966	3	32.378	5	0	2	.117	1	0	15
496			min	-14.748	5	-369.071	2	-49.136	1	0	3	-.082	5	-.012	2
497		2	max	122.721	1	706.086	3	33.62	5	0	2	.091	1	.183	2
498			min	-14.364	5	-370.245	2	-49.136	1	0	3	-.065	5	-.375	3
499		3	max	597.993	3	484.981	2	20.899	5	0	3	.065	1	.368	2
500			min	-344.91	2	-552.387	3	-49.027	1	0	2	-.047	5	-.732	3
501		4	max	598.609	3	483.808	2	22.14	5	0	3	.039	1	.112	2
502			min	-344.089	2	-553.267	3	-49.027	1	0	2	-.036	5	-.44	3
503		5	max	599.226	3	482.634	2	23.382	5	0	3	.013	1	-.003	15
504			min	-343.267	2	-554.147	3	-49.027	1	0	2	-.024	5	-.148	3
505		6	max	599.842	3	481.461	2	24.623	5	0	3	-.001	12	.145	3
506			min	-342.445	2	-555.027	3	-49.027	1	0	2	-.014	4	-.397	2
507		7	max	600.458	3	480.288	2	25.865	5	0	3	.002	5	.438	3
508			min	-341.624	2	-555.907	3	-49.027	1	0	2	-.039	1	-.651	2
509		8	max	601.074	3	479.114	2	27.106	5	0	3	.016	5	.731	3
510			min	-340.802	2	-556.787	3	-49.027	1	0	2	-.064	1	-.904	2
511		9	max	616.115	3	52.433	2	49.032	5	0	9	.041	1	.849	3
512			min	-286.669	2	.354	15	-77.726	1	0	3	-.096	5	-1.033	2
513		10	max	616.732	3	51.26	2	50.274	5	0	9	0	10	.832	3
514			min	-285.848	2	0	5	-77.726	1	0	3	-.071	4	-1.061	2
515		11	max	617.348	3	50.086	2	51.515	5	0	9	-.005	10	.815	3
516			min	-285.026	2	-1.476	4	-77.726	1	0	3	-.054	4	-1.087	2
517		12	max	632.078	3	384.019	3	122.344	5	0	2	.064	1	.715	3
518			min	-230.746	2	-589.775	2	-48.267	1	0	3	-.182	5	-.966	2
519		13	max	632.694	3	383.139	3	123.586	5	0	2	.038	1	.512	3
520			min	-229.924	2	-590.948	2	-48.267	1	0	3	-.117	5	-.655	2
521		14	max	633.31	3	382.259	3	124.827	5	0	2	.013	1	.31	3
522			min	-229.103	2	-592.122	2	-48.267	1	0	3	-.052	5	-.343	2
523		15	max	633.926	3	381.379	3	126.069	5	0	2	.014	5	.109	3
524			min	-228.281	2	-593.295	2	-48.267	1	0	3	-.013	1	-.039	1
525		16	max	634.543	3	380.499	3	127.31	5	0	2	.081	5	.283	2
526			min	-227.459	2	-594.469	2	-48.267	1	0	3	-.038	1	-.092	3
527		17	max	635.159	3	379.619	3	128.552	5	0	2	.149	5	.597	2
528			min	-226.638	2	-595.642	2	-48.267	1	0	3	-.063	1	-.293	3
529		18	max	28.433	5	586.071	2	-5.635	12	0	5	.172	5	.302	2
530			min	-123.14	1	-295.966	3	-87.113	4	0	2	-.09	1	-.145	3
531		19	max	28.817	5	584.898	2	-5.635	12	0	5	.134	5	.011	3





Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-122.319	1	-296.846	3	-85.871	4	0	2	-.118	1	-.007	2
533	M5	max	274.913	1	2337.771	3	70.301	5	0	1	0	1	.025	2
534		min	10.398	12	-1278.202	2	0	1	0	4	-.17	4	0	15
535		max	275.734	1	2336.891	3	71.543	5	0	1	0	1	.7	2
536		min	10.809	12	-1279.375	2	0	1	0	4	-.133	4	-1.23	3
537		max	1841.372	3	1322.053	2	60.008	4	0	4	0	1	1.344	2
538		min	-1093.649	2	-1630.552	3	0	1	0	1	-.095	4	-2.415	3
539		max	1841.988	3	1320.88	2	61.25	4	0	4	0	1	.646	2
540		min	-1092.827	2	-1631.432	3	0	1	0	1	-.063	4	-1.554	3
541		max	1842.604	3	1319.706	2	62.491	4	0	4	0	1	.015	9
542		min	-1092.006	2	-1632.312	3	0	1	0	1	-.03	4	-.693	3
543		max	1843.22	3	1318.533	2	63.732	4	0	4	.003	4	.168	3
544		min	-1091.184	2	-1633.192	3	0	1	0	1	0	1	-.746	2
545		max	1843.837	3	1317.359	2	64.974	4	0	4	.037	4	1.03	3
546		min	-1090.363	2	-1634.072	3	0	1	0	1	0	1	-1.442	2
547		max	1844.453	3	1316.186	2	66.215	4	0	4	.072	4	1.893	3
548		min	-1089.541	2	-1634.952	3	0	1	0	1	0	1	-2.137	2
549		max	1859.25	3	177.875	2	165.28	4	0	1	0	1	2.175	3
550		min	-967.857	2	.35	15	0	1	0	1	-.15	4	-2.447	2
551		max	1859.866	3	176.701	2	166.521	4	0	1	0	1	2.108	3
552		min	-967.035	2	-.004	15	0	1	0	1	-.063	4	-2.54	2
553		max	1860.482	3	175.528	2	167.763	4	0	1	.025	4	2.042	3
554		min	-966.213	2	-1.433	6	0	1	0	1	0	1	-2.633	2
555		max	1875.902	3	1095.813	3	178.726	4	0	1	0	1	1.791	3
556		min	-844.824	2	-1673.989	2	0	1	0	4	-.263	4	-2.361	2
557		max	1876.518	3	1094.933	3	179.967	4	0	1	0	1	1.213	3
558		min	-844.002	2	-1675.162	2	0	1	0	4	-.169	4	-1.478	2
559		max	1877.135	3	1094.053	3	181.209	4	0	1	0	1	.636	3
560		min	-843.181	2	-1676.335	2	0	1	0	4	-.074	4	-.593	2
561		max	1877.751	3	1093.172	3	182.45	4	0	1	.022	4	.291	2
562		min	-842.359	2	-1677.509	2	0	1	0	4	0	1	0	13
563		max	1878.367	3	1092.292	3	183.692	4	0	1	.119	4	1.177	2
564		min	-841.538	2	-1678.682	2	0	1	0	4	0	1	-.518	3
565		max	1878.983	3	1091.412	3	184.933	4	0	1	.216	4	2.063	2
566		min	-840.716	2	-1679.856	2	0	1	0	4	0	1	-1.094	3
567		max	-12.1	12	1983.564	2	0	1	0	4	.27	4	1.06	2
568		min	-274.896	1	-1039.258	3	-11.569	5	0	1	0	1	-.571	3
569		max	-11.689	12	1982.391	2	0	1	0	4	.265	4	.014	2
570		min	-274.075	1	-1040.138	3	-10.328	5	0	1	0	1	-.023	3
571	M9	max	121.899	1	706.966	3	54.14	4	0	3	-.013	10	0	15
572		min	9.145	12	-369.071	2	5.375	10	0	4	-.135	4	-.012	2
573		max	122.721	1	706.086	3	55.381	4	0	3	-.01	10	.183	2
574		min	9.556	12	-370.245	2	5.375	10	0	4	-.106	4	-.375	3
575		max	597.993	3	484.981	2	49.027	1	0	2	-.007	10	.368	2
576		min	-344.91	2	-552.387	3	5.358	10	0	3	-.076	4	-.732	3
577		max	598.609	3	483.808	2	49.027	1	0	2	-.004	10	.112	2
578		min	-344.089	2	-553.267	3	5.358	10	0	3	-.053	4	-.44	3
579		max	599.226	3	482.634	2	49.027	1	0	2	-.001	10	-.003	15
580		min	-343.267	2	-554.147	3	5.358	10	0	3	-.03	4	-.148	3
581		max	599.842	3	481.461	2	49.027	1	0	2	.013	1	.145	3
582		min	-342.445	2	-555.027	3	5.358	10	0	3	-.009	5	-.397	2
583		max	600.458	3	480.288	2	49.027	1	0	2	.039	1	.438	3
584		min	-341.624	2	-555.907	3	5.358	10	0	3	.004	10	-.651	2
585		max	601.074	3	479.114	2	49.127	4	0	2	.064	1	.731	3
586		min	-340.802	2	-556.787	3	5.358	10	0	3	.007	10	-.904	2
587		max	616.115	3	52.433	2	83.52	4	0	3	-.004	12	.849	3
588		min	-286.669	2	.363	15	8.791	12	0	9	-.115	4	-1.033	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	616.732	3	51.26	2	84.761	4	0	3	0	1	.832	3
590		min	-285.848	2	.009	15	8.791	12	0	9	-.07	4	-1.061	2
591	11	max	617.348	3	50.086	2	86.003	4	0	3	.041	1	.815	3
592		min	-285.026	2	-1.424	6	8.791	12	0	9	-.036	5	-1.087	2
593	12	max	632.078	3	384.019	3	144.388	4	0	3	-.007	12	.715	3
594		min	-230.746	2	-589.775	2	5.074	12	0	2	-.212	4	-.966	2
595	13	max	632.694	3	383.139	3	145.63	4	0	3	-.004	12	.512	3
596		min	-229.924	2	-590.948	2	5.074	12	0	2	-.135	4	-.655	2
597	14	max	633.31	3	382.259	3	146.871	4	0	3	-.001	10	.31	3
598		min	-229.103	2	-592.122	2	5.074	12	0	2	-.058	4	-.343	2
599	15	max	633.926	3	381.379	3	148.113	4	0	3	.02	4	.109	3
600		min	-228.281	2	-593.295	2	5.074	12	0	2	.001	12	-.039	1
601	16	max	634.543	3	380.499	3	149.354	4	0	3	.098	4	.283	2
602		min	-227.459	2	-594.469	2	5.074	12	0	2	.004	12	-.092	3
603	17	max	635.159	3	379.619	3	150.596	4	0	3	.178	4	.597	2
604		min	-226.638	2	-595.642	2	5.074	12	0	2	.007	12	-.293	3
605	18	max	-8.91	12	586.071	2	53.717	1	0	2	.213	4	.302	2
606		min	-123.14	1	-295.966	3	-63.292	5	0	3	.009	12	-.145	3
607	19	max	-8.499	12	584.898	2	53.717	1	0	2	.187	4	.011	3
608		min	-122.319	1	-296.846	3	-62.051	5	0	3	.012	12	-.007	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.112	2	.01	3	9.512e-3	2	NC	1	NC	1
2				min	-445	4	-.031	3	-.006	2	-3.06e-3	3	NC	1	NC
3		2	max	0	1	.088	3	.012	3	1.036e-2	2	NC	4	NC	1
4			min	-445	4	.002	15	-.008	5	-2.939e-3	3	1411.468	3	NC	1
5		3	max	0	1	.185	3	.023	1	1.122e-2	2	NC	4	NC	2
6			min	-445	4	0	9	-.01	5	-2.818e-3	3	777.087	3	6906.683	1
7		4	max	0	1	.246	3	.035	1	1.207e-2	2	NC	4	NC	2
8			min	-445	4	-.005	9	-.008	5	-2.696e-3	3	606.988	3	4711.209	1
9		5	max	0	1	.263	3	.04	1	1.292e-2	2	NC	4	NC	2
10			min	-445	4	-.005	9	-.002	5	-2.575e-3	3	571.747	3	4124.064	1
11		6	max	0	1	.237	3	.037	1	1.377e-2	2	NC	4	NC	2
12			min	-445	4	0	15	-.004	10	-2.454e-3	3	626.37	3	4423.746	1
13		7	max	0	1	.178	3	.028	3	1.463e-2	2	NC	4	NC	2
14			min	-445	4	.002	15	-.006	10	-2.332e-3	3	805.652	3	6007.605	1
15		8	max	0	1	.135	2	.03	3	1.548e-2	2	NC	1	NC	1
16			min	-445	4	.003	15	-.011	2	-2.211e-3	3	1280.632	3	8652.928	3
17		9	max	0	1	.178	2	.03	3	1.633e-2	2	NC	4	NC	1
18			min	-445	4	.003	15	-.018	2	-2.089e-3	3	2544.55	2	8411.001	3
19		10	max	0	1	.197	2	.03	3	1.718e-2	2	NC	4	NC	1
20			min	-445	4	-.003	3	-.022	2	-1.968e-3	3	1971.306	2	8371.587	3
21		11	max	0	10	.178	2	.03	3	1.633e-2	2	NC	4	NC	1
22			min	-445	4	.003	15	-.018	2	-2.089e-3	3	2544.55	2	8411.001	3
23		12	max	0	10	.135	2	.03	3	1.548e-2	2	NC	1	NC	1
24			min	-445	4	.002	15	-.011	2	-2.211e-3	3	1280.632	3	8652.928	3
25		13	max	0	10	.178	3	.028	3	1.463e-2	2	NC	4	NC	2
26			min	-445	4	.001	15	-.006	10	-2.332e-3	3	805.652	3	6007.605	1
27		14	max	0	10	.237	3	.037	1	1.377e-2	2	NC	4	NC	2
28			min	-445	4	0	15	-.004	10	-2.454e-3	3	626.37	3	4423.746	1
29		15	max	0	10	.263	3	.04	1	1.292e-2	2	NC	4	NC	2
30			min	-445	4	-.005	9	-.002	10	-2.575e-3	3	571.747	3	4124.064	1
31		16	max	0	10	.246	3	.035	1	1.207e-2	2	NC	4	NC	2
32			min	-445	4	-.005	9	-.001	10	-2.696e-3	3	606.988	3	4711.209	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	10	.185	3	.023	1	1.122e-2	2	NC	4	NC	2
34		min	-.445	4	0	15	-.002	10	-2.818e-3	3	777.087	3	6906.683	1
35	18	max	0	10	.088	3	.014	4	1.036e-2	2	NC	4	NC	1
36		min	-.445	4	0	15	-.003	10	-2.939e-3	3	1411.468	3	NC	1
37	19	max	0	10	.112	2	.01	3	9.512e-3	2	NC	1	NC	1
38		min	-.445	4	-.031	3	-.006	2	-3.06e-3	3	NC	1	NC	1
39	M14	1	max	0	.265	3	.009	3	5.217e-3	2	NC	1	NC	1
40		min	-.34	4	-.353	2	-.006	2	-4.447e-3	3	NC	1	NC	1
41	2	max	0	1	.416	3	.01	3	6.022e-3	2	NC	4	NC	1
42		min	-.34	4	-.49	2	-.013	5	-5.204e-3	3	1110.529	3	NC	1
43	3	max	0	1	.549	3	.018	1	6.827e-3	2	NC	5	NC	2
44		min	-.34	4	-.613	2	-.016	5	-5.961e-3	3	591.189	3	9091.094	1
45	4	max	0	1	.651	3	.028	1	7.632e-3	2	NC	5	NC	2
46		min	-.34	4	-.713	2	-.011	5	-6.717e-3	3	435.438	3	5732.263	1
47	5	max	0	1	.714	3	.034	1	8.437e-3	2	NC	5	NC	2
48		min	-.34	4	-.785	2	-.002	5	-7.474e-3	3	373.851	3	4801.781	1
49	6	max	0	1	.739	3	.033	1	9.242e-3	2	NC	5	NC	2
50		min	-.34	4	-.826	2	-.003	10	-8.231e-3	3	354.404	3	5005.389	1
51	7	max	0	1	.73	3	.025	14	1.005e-2	2	NC	5	NC	2
52		min	-.34	4	-.84	2	-.006	10	-8.988e-3	3	345.058	2	6654.344	1
53	8	max	0	1	.698	3	.026	3	1.085e-2	2	NC	5	NC	1
54		min	-.34	4	-.833	2	-.01	2	-9.745e-3	3	349.803	2	7177.994	4
55	9	max	0	1	.662	3	.027	3	1.166e-2	2	NC	5	NC	1
56		min	-.34	4	-.817	2	-.017	2	-1.05e-2	3	361.652	2	9487.506	3
57	10	max	0	1	.643	3	.027	3	1.246e-2	2	NC	5	NC	1
58		min	-.34	4	-.808	2	-.02	2	-1.126e-2	3	369.175	2	9426.223	3
59	11	max	0	12	.662	3	.027	3	1.166e-2	2	NC	5	NC	1
60		min	-.34	4	-.817	2	-.017	2	-1.05e-2	3	361.652	2	9487.506	3
61	12	max	0	12	.698	3	.026	3	1.085e-2	2	NC	5	NC	1
62		min	-.34	4	-.833	2	-.015	5	-9.745e-3	3	349.803	2	9817.861	3
63	13	max	0	12	.73	3	.025	3	1.005e-2	2	NC	5	NC	2
64		min	-.34	4	-.84	2	-.01	5	-8.988e-3	3	345.058	2	6654.344	1
65	14	max	0	12	.739	3	.033	1	9.242e-3	2	NC	5	NC	2
66		min	-.34	4	-.826	2	-.003	10	-8.231e-3	3	354.404	3	5005.389	1
67	15	max	0	12	.714	3	.034	1	8.437e-3	2	NC	5	NC	2
68		min	-.34	4	-.785	2	-.002	10	-7.474e-3	3	373.851	3	4801.781	1
69	16	max	0	12	.651	3	.028	1	7.632e-3	2	NC	5	NC	2
70		min	-.34	4	-.713	2	-.002	10	-6.717e-3	3	435.438	3	5732.263	1
71	17	max	0	12	.549	3	.026	4	6.827e-3	2	NC	5	NC	2
72		min	-.34	4	-.613	2	-.002	10	-5.961e-3	3	591.189	3	6479.752	4
73	18	max	0	12	.416	3	.017	4	6.022e-3	2	NC	4	NC	1
74		min	-.34	4	-.49	2	-.003	2	-5.204e-3	3	1110.529	3	9562.624	4
75	19	max	0	12	.265	3	.009	3	5.217e-3	2	NC	1	NC	1
76		min	-.34	4	-.353	2	-.006	2	-4.447e-3	3	NC	1	NC	1
77	M15	1	max	0	.269	3	.008	3	3.958e-3	3	NC	1	NC	1
78		min	-.282	4	-.352	2	-.005	2	-5.492e-3	2	NC	1	NC	1
79	2	max	0	10	.382	3	.01	3	4.633e-3	3	NC	4	NC	1
80		min	-.282	4	-.523	2	-.018	5	-6.347e-3	2	980.228	2	8627.033	5
81	3	max	0	10	.484	3	.018	1	5.308e-3	3	NC	5	NC	2
82		min	-.282	4	-.674	2	-.023	5	-7.203e-3	2	520.674	2	7027.858	5
83	4	max	0	10	.567	3	.029	1	5.984e-3	3	NC	5	NC	2
84		min	-.282	4	-.791	2	-.017	5	-8.058e-3	2	382.097	2	5703.551	1
85	5	max	0	10	.627	3	.034	1	6.659e-3	3	NC	5	NC	2
86		min	-.282	4	-.866	2	-.005	5	-8.914e-3	2	326.306	2	4774.341	1
87	6	max	0	10	.662	3	.033	1	7.335e-3	3	NC	5	NC	2
88		min	-.282	4	-.899	2	-.003	10	-9.769e-3	2	307.064	2	4968.127	1
89	7	max	0	10	.675	3	.028	4	8.01e-3	3	NC	5	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.282	4	-.893	2	-.005	10	-1.062e-2	2	309.998	2	6306.326	4
91	8	max	0	10	.671	3	.03	4	8.685e-3	3	NC	5	NC	1
92		min	-.282	4	-.863	2	-.009	2	-1.148e-2	2	328.561	2	5872.721	4
93	9	max	0	10	.659	3	.025	3	9.361e-3	3	NC	5	NC	1
94		min	-.282	4	-.825	2	-.016	2	-1.234e-2	2	354.581	2	8227.551	4
95	10	max	0	1	.651	3	.025	3	1.004e-2	3	NC	5	NC	1
96		min	-.282	4	-.806	2	-.019	2	-1.319e-2	2	369.687	2	NC	1
97	11	max	0	1	.659	3	.025	3	9.361e-3	3	NC	5	NC	1
98		min	-.282	4	-.825	2	-.017	5	-1.234e-2	2	354.581	2	9564.483	5
99	12	max	0	1	.671	3	.024	3	8.685e-3	3	NC	5	NC	1
100		min	-.282	4	-.863	2	-.021	5	-1.148e-2	2	328.561	2	8170.846	5
101	13	max	0	1	.675	3	.025	1	8.01e-3	3	NC	5	NC	2
102		min	-.282	4	-.893	2	-.014	5	-1.062e-2	2	309.998	2	6577.113	1
103	14	max	0	1	.662	3	.033	1	7.335e-3	3	NC	5	NC	2
104		min	-.282	4	-.899	2	-.003	10	-9.769e-3	2	307.064	2	4968.127	1
105	15	max	0	1	.627	3	.034	1	6.659e-3	3	NC	5	NC	2
106		min	-.282	4	-.866	2	-.001	10	-8.914e-3	2	326.306	2	4774.341	1
107	16	max	0	1	.567	3	.031	4	5.984e-3	3	NC	5	NC	2
108		min	-.282	4	-.791	2	-.001	10	-8.058e-3	2	382.097	2	5362.809	4
109	17	max	0	1	.484	3	.032	4	5.308e-3	3	NC	5	NC	2
110		min	-.282	4	-.674	2	-.002	10	-7.203e-3	2	520.674	2	5192.136	4
111	18	max	0	1	.382	3	.022	4	4.633e-3	3	NC	4	NC	1
112		min	-.282	4	-.523	2	-.003	10	-6.347e-3	2	980.228	2	7443.311	4
113	19	max	0	1	.269	3	.008	3	3.958e-3	3	NC	1	NC	1
114		min	-.282	4	-.352	2	-.005	2	-5.492e-3	2	NC	1	NC	1
115	M16	1	max	0	.099	2	.007	3	7.304e-3	3	NC	1	NC	1
116		min	-.106	4	-.09	3	-.005	2	-7.811e-3	2	NC	1	NC	1
117	2	max	0	12	.02	1	.01	1	8.083e-3	3	NC	4	NC	1
118		min	-.106	4	-.055	3	-.014	5	-8.284e-3	2	1947.273	2	NC	1
119	3	max	0	12	.004	4	.024	1	8.862e-3	3	NC	4	NC	2
120		min	-.106	4	-.056	2	-.018	5	-8.757e-3	2	1087.113	2	6876.464	1
121	4	max	0	12	.002	13	.035	1	9.642e-3	3	NC	4	NC	2
122		min	-.106	4	-.094	2	-.015	5	-9.23e-3	2	871.519	2	4670.652	1
123	5	max	0	12	.002	13	.04	1	1.042e-2	3	NC	4	NC	2
124		min	-.106	4	-.096	2	-.007	5	-9.703e-3	2	859.458	2	4067.23	1
125	6	max	0	12	.005	4	.038	1	1.12e-2	3	NC	4	NC	2
126		min	-.106	4	-.065	2	-.002	10	-1.018e-2	2	1026.289	2	4326.498	1
127	7	max	0	12	.014	9	.028	1	1.198e-2	3	NC	3	NC	2
128		min	-.107	4	-.084	3	-.004	10	-1.065e-2	2	1599.742	2	5774.127	1
129	8	max	0	12	.065	2	.021	3	1.276e-2	3	NC	1	NC	1
130		min	-.107	4	-.126	3	-.007	10	-1.112e-2	2	4619.138	3	8526.301	4
131	9	max	0	12	.129	2	.022	3	1.354e-2	3	NC	4	NC	1
132		min	-.107	4	-.162	3	-.014	2	-1.16e-2	2	2323.511	3	NC	1
133	10	max	0	1	.158	2	.021	3	1.432e-2	3	NC	4	NC	1
134		min	-.107	4	-.178	3	-.017	2	-1.207e-2	2	1905.238	3	NC	1
135	11	max	0	1	.129	2	.022	3	1.354e-2	3	NC	4	NC	1
136		min	-.106	4	-.162	3	-.014	2	-1.16e-2	2	2323.511	3	NC	1
137	12	max	0	1	.065	2	.021	3	1.276e-2	3	NC	1	NC	1
138		min	-.106	4	-.126	3	-.011	5	-1.112e-2	2	4619.138	3	NC	1
139	13	max	0	1	.014	9	.028	1	1.198e-2	3	NC	3	NC	2
140		min	-.106	4	-.084	3	-.005	5	-1.065e-2	2	1599.742	2	5774.127	1
141	14	max	0	1	.004	9	.038	1	1.12e-2	3	NC	4	NC	2
142		min	-.106	4	-.065	2	-.002	10	-1.018e-2	2	1026.289	2	4326.498	1
143	15	max	0	1	.001	13	.04	1	1.042e-2	3	NC	4	NC	2
144		min	-.106	4	-.096	2	0	10	-9.703e-3	2	859.458	2	4067.23	1
145	16	max	0	1	.001	13	.035	1	9.642e-3	3	NC	4	NC	2
146		min	-.106	4	-.094	2	0	10	-9.23e-3	2	871.519	2	4670.652	1



Company : Schletter, Inc.  
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Job Number :  
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Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147	17	max	0	1	.003	6	.03	4	8.862e-3	3	NC	4	NC	2
148		min	-.106	4	-.056	2	0	10	-8.757e-3	2	1087.113	2	5516.987	4
149	18	max	0	1	.02	1	.02	4	8.083e-3	3	NC	4	NC	1
150		min	-.106	4	-.055	3	-.002	10	-8.284e-3	2	1947.273	2	8349.285	4
151	19	max	0	1	.099	2	.007	3	7.304e-3	3	NC	1	NC	1
152		min	-.106	4	-.09	3	-.005	2	-7.811e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.01	.004	1	1.301e-3	5	NC	1	NC	1
154		min	-.01	3	-.016	3	-.42	4	-1.029e-4	1	7453.502	2	183.489	4
155	2	max	.007	2	.009	2	.004	1	1.334e-3	5	NC	1	NC	1
156		min	-.01	3	-.015	3	-.386	4	-9.758e-5	1	8642.116	2	199.387	4
157	3	max	.006	2	.008	2	.003	1	1.366e-3	5	NC	1	NC	1
158		min	-.009	3	-.015	3	-.353	4	-9.23e-5	1	NC	1	218.196	4
159	4	max	.006	2	.006	2	.003	1	1.399e-3	5	NC	1	NC	1
160		min	-.009	3	-.014	3	-.32	4	-8.702e-5	1	NC	1	240.669	4
161	5	max	.005	2	.005	2	.003	1	1.431e-3	5	NC	1	NC	1
162		min	-.008	3	-.014	3	-.288	4	-8.174e-5	1	NC	1	267.819	4
163	6	max	.005	2	.004	2	.002	1	1.464e-3	5	NC	1	NC	1
164		min	-.007	3	-.013	3	-.256	4	-7.646e-5	1	NC	1	301.046	4
165	7	max	.005	2	.002	2	.002	1	1.496e-3	5	NC	1	NC	1
166		min	-.007	3	-.012	3	-.225	4	-7.117e-5	1	NC	1	342.326	4
167	8	max	.004	2	.001	2	.002	1	1.529e-3	5	NC	1	NC	1
168		min	-.006	3	-.012	3	-.195	4	-6.589e-5	1	NC	1	394.525	4
169	9	max	.004	2	0	2	.001	1	1.561e-3	5	NC	1	NC	1
170		min	-.006	3	-.011	3	-.167	4	-6.061e-5	1	NC	1	461.927	4
171	10	max	.004	2	0	2	.001	1	1.594e-3	5	NC	1	NC	1
172		min	-.005	3	-.01	3	-.14	4	-5.533e-5	1	NC	1	551.177	4
173	11	max	.003	2	0	2	0	1	1.626e-3	5	NC	1	NC	1
174		min	-.005	3	-.009	3	-.114	4	-5.005e-5	1	NC	1	673.047	4
175	12	max	.003	2	-.001	15	0	1	1.659e-3	5	NC	1	NC	1
176		min	-.004	3	-.008	3	-.091	4	-4.476e-5	1	NC	1	845.967	4
177	13	max	.002	2	-.001	15	0	1	1.692e-3	4	NC	1	NC	1
178		min	-.003	3	-.007	3	-.07	4	-3.948e-5	1	NC	1	1103.685	4
179	14	max	.002	2	0	15	0	1	1.726e-3	4	NC	1	NC	1
180		min	-.003	3	-.006	3	-.051	4	-3.42e-5	1	NC	1	1513.685	4
181	15	max	.002	2	0	15	0	1	1.76e-3	4	NC	1	NC	1
182		min	-.002	3	-.005	3	-.035	4	-2.892e-5	1	NC	1	2228.026	4
183	16	max	.001	2	0	15	0	1	1.794e-3	4	NC	1	NC	1
184		min	-.002	3	-.004	3	-.021	4	-2.364e-5	1	NC	1	3653.795	4
185	17	max	0	2	0	15	0	1	1.828e-3	4	NC	1	NC	1
186		min	-.001	3	-.003	3	-.011	4	-1.835e-5	1	NC	1	7227.753	4
187	18	max	0	2	0	15	0	1	1.863e-3	4	NC	1	NC	1
188		min	0	3	-.001	3	-.004	4	-1.307e-5	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.897e-3	4	NC	1	NC	1
190		min	0	1	0	1	0	1	-7.791e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	1	1.782e-6	1	NC	1	NC	1
192		min	0	1	0	1	0	1	-4.87e-4	4	NC	1	NC	1
193	2	max	0	3	0	15	.009	4	1.148e-5	1	NC	1	NC	1
194		min	0	2	-.002	6	0	1	-4.858e-5	5	NC	1	NC	1
195	3	max	0	3	0	15	.017	4	3.969e-4	4	NC	1	NC	1
196		min	0	2	-.004	6	0	1	2.194e-6	12	NC	1	5312.839	4
197	4	max	.001	3	-.001	15	.024	4	3.389e-4	4	NC	1	NC	1
198		min	-.001	2	-.006	6	0	1	3.139e-6	12	NC	1	3713.353	4
199	5	max	.002	3	-.002	15	.031	4	1.281e-3	4	NC	1	NC	1
200		min	-.002	2	-.008	6	0	1	4.084e-6	12	NC	1	2912.421	4
201	6	max	.002	3	-.002	15	.037	4	1.723e-3	4	NC	1	NC	1
202		min	-.002	2	-.01	6	0	1	5.028e-6	12	9378.374	6	2428.412	4
203	7	max	.003	3	-.002	15	.043	4	2.165e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.011	6	0	3	5.973e-6	12	8101.997	6	2100.38	4
205		8	max	.003	3	-.003	15	.048	4	2.607e-3	4	NC	2	NC	1
206			min	-.003	2	-.012	6	0	3	6.918e-6	12	7316.181	6	1859.06	4
207		9	max	.004	3	-.003	15	.054	4	3.049e-3	4	NC	5	NC	1
208			min	-.003	2	-.013	6	0	12	7.863e-6	12	6856.848	6	1669.678	4
209		10	max	.004	3	-.003	15	.059	4	3.491e-3	4	NC	5	NC	1
210			min	-.004	2	-.014	6	0	12	8.808e-6	12	6644.91	6	1512.93	4
211		11	max	.005	3	-.003	15	.065	4	3.933e-3	4	NC	5	NC	1
212			min	-.004	2	-.013	6	0	12	9.753e-6	12	6649.399	6	1377.411	4
213		12	max	.005	3	-.003	15	.072	4	4.375e-3	4	NC	5	NC	1
214			min	-.005	2	-.013	6	0	12	1.07e-5	12	6875.596	6	1256.191	4
215		13	max	.005	3	-.003	15	.078	4	4.817e-3	4	NC	2	NC	1
216			min	-.005	2	-.012	6	0	12	1.164e-5	12	7368.591	6	1145.082	4
217		14	max	.006	3	-.002	15	.086	4	5.259e-3	4	NC	1	NC	1
218			min	-.005	2	-.011	6	0	12	1.259e-5	12	8236.178	6	1041.671	4
219		15	max	.006	3	-.002	15	.095	4	5.7e-3	4	NC	1	NC	1
220			min	-.006	2	-.009	6	0	12	1.353e-5	12	9715.428	6	944.715	4
221		16	max	.007	3	-.001	15	.105	4	6.142e-3	4	NC	1	NC	1
222			min	-.006	2	-.007	6	0	12	1.448e-5	12	NC	1	853.709	4
223		17	max	.007	3	0	15	.117	4	6.584e-3	4	NC	1	NC	1
224			min	-.007	2	-.006	3	0	12	1.542e-5	12	NC	1	768.575	4
225		18	max	.008	3	0	15	.13	4	7.026e-3	4	NC	1	NC	1
226			min	-.007	2	-.004	3	0	12	1.637e-5	12	NC	1	689.43	4
227		19	max	.008	3	0	2	.146	4	7.468e-3	4	NC	1	NC	1
228			min	-.007	2	-.003	3	0	12	1.731e-5	12	NC	1	616.422	4
229	M4	1	max	.002	1	.007	2	0	12	1.123e-3	4	NC	1	NC	2
230			min	0	5	-.009	3	-.146	4	7.717e-6	12	NC	1	170.2	4
231		2	max	.002	1	.007	2	0	12	1.123e-3	4	NC	1	NC	2
232			min	0	5	-.008	3	-.134	4	7.717e-6	12	NC	1	184.505	4
233		3	max	.002	1	.006	2	0	12	1.123e-3	4	NC	1	NC	2
234			min	0	5	-.008	3	-.123	4	7.717e-6	12	NC	1	201.565	4
235		4	max	.001	1	.006	2	0	12	1.123e-3	4	NC	1	NC	2
236			min	0	5	-.007	3	-.112	4	7.717e-6	12	NC	1	222.089	4
237		5	max	.001	1	.006	2	0	12	1.123e-3	4	NC	1	NC	2
238			min	0	5	-.007	3	-.1	4	7.717e-6	12	NC	1	247.043	4
239		6	max	.001	1	.005	2	0	12	1.123e-3	4	NC	1	NC	1
240			min	0	5	-.006	3	-.089	4	7.717e-6	12	NC	1	277.763	4
241		7	max	.001	1	.005	2	0	12	1.123e-3	4	NC	1	NC	1
242			min	0	5	-.006	3	-.078	4	7.717e-6	12	NC	1	316.149	4
243		8	max	.001	1	.004	2	0	12	1.123e-3	4	NC	1	NC	1
244			min	0	5	-.005	3	-.068	4	7.717e-6	12	NC	1	364.968	4
245		9	max	0	1	.004	2	0	12	1.123e-3	4	NC	1	NC	1
246			min	0	5	-.005	3	-.058	4	7.717e-6	12	NC	1	428.386	4
247		10	max	0	1	.004	2	0	12	1.123e-3	4	NC	1	NC	1
248			min	0	5	-.004	3	-.048	4	7.717e-6	12	NC	1	512.916	4
249		11	max	0	1	.003	2	0	12	1.123e-3	4	NC	1	NC	1
250			min	0	5	-.004	3	-.039	4	7.717e-6	12	NC	1	629.201	4
251		12	max	0	1	.003	2	0	12	1.123e-3	4	NC	1	NC	1
252			min	0	5	-.003	3	-.031	4	7.717e-6	12	NC	1	795.649	4
253		13	max	0	1	.002	2	0	12	1.123e-3	4	NC	1	NC	1
254			min	0	5	-.003	3	-.024	4	7.717e-6	12	NC	1	1046.401	4
255		14	max	0	1	.002	2	0	12	1.123e-3	4	NC	1	NC	1
256			min	0	5	-.002	3	-.017	4	7.717e-6	12	NC	1	1450.871	4
257		15	max	0	1	.002	2	0	12	1.123e-3	4	NC	1	NC	1
258			min	0	5	-.002	3	-.011	4	7.717e-6	12	NC	1	2168.936	4
259		16	max	0	1	.001	2	0	12	1.123e-3	4	NC	1	NC	1
260			min	0	5	-.001	3	-.007	4	7.717e-6	12	NC	1	3642.094	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	1.123e-3	4	NC	1	NC	1
262			min	0	5	0	3	-.003	4	7.717e-6	12	NC	1	7504.877	4
263		18	max	0	1	0	2	0	12	1.123e-3	4	NC	1	NC	1
264			min	0	5	0	3	-.001	4	7.717e-6	12	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.123e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	7.717e-6	12	NC	1	NC	1
267	M6	1	max	.022	2	.034	2	0	1	1.355e-3	4	NC	4	NC	1
268			min	-.032	3	-.048	3	-.423	4	0	1	1599.027	3	181.887	4
269		2	max	.02	2	.031	2	0	1	1.386e-3	4	NC	4	NC	1
270			min	-.03	3	-.045	3	-.39	4	0	1	1692.785	3	197.648	4
271		3	max	.019	2	.028	2	0	1	1.418e-3	4	NC	4	NC	1
272			min	-.029	3	-.043	3	-.356	4	0	1	1798.361	3	216.296	4
273		4	max	.018	2	.025	2	0	1	1.449e-3	4	NC	4	NC	1
274			min	-.027	3	-.04	3	-.323	4	0	1	1918.265	3	238.575	4
275		5	max	.017	2	.022	2	0	1	1.48e-3	4	NC	4	NC	1
276			min	-.025	3	-.037	3	-.29	4	0	1	2055.718	3	265.491	4
277		6	max	.016	2	.019	2	0	1	1.512e-3	4	NC	4	NC	1
278			min	-.023	3	-.035	3	-.258	4	0	1	2214.94	3	298.432	4
279		7	max	.014	2	.017	2	0	1	1.543e-3	4	NC	1	NC	1
280			min	-.021	3	-.032	3	-.227	4	0	1	2401.554	3	339.358	4
281		8	max	.013	2	.014	2	0	1	1.574e-3	4	NC	1	NC	1
282			min	-.02	3	-.029	3	-.197	4	0	1	2623.234	3	391.107	4
283		9	max	.012	2	.012	2	0	1	1.606e-3	4	NC	1	NC	1
284			min	-.018	3	-.027	3	-.168	4	0	1	2890.725	3	457.928	4
285		10	max	.011	2	.01	2	0	1	1.637e-3	4	NC	1	NC	1
286			min	-.016	3	-.024	3	-.141	4	0	1	3219.562	3	546.407	4
287		11	max	.01	2	.008	2	0	1	1.669e-3	4	NC	1	NC	1
288			min	-.014	3	-.021	3	-.115	4	0	1	3633.052	3	667.22	4
289		12	max	.008	2	.006	2	0	1	1.7e-3	4	NC	1	NC	1
290			min	-.012	3	-.018	3	-.092	4	0	1	4167.846	3	838.635	4
291		13	max	.007	2	.004	2	0	1	1.731e-3	4	NC	1	NC	1
292			min	-.011	3	-.016	3	-.07	4	0	1	4885.05	3	1094.096	4
293		14	max	.006	2	.003	2	0	1	1.763e-3	4	NC	1	NC	1
294			min	-.009	3	-.013	3	-.051	4	0	1	5894.703	3	1500.478	4
295		15	max	.005	2	.002	2	0	1	1.794e-3	4	NC	1	NC	1
296			min	-.007	3	-.01	3	-.035	4	0	1	7416.943	3	2208.442	4
297		16	max	.004	2	0	2	0	1	1.825e-3	4	NC	1	NC	1
298			min	-.005	3	-.008	3	-.021	4	0	1	9965.532	3	3621.263	4
299		17	max	.002	2	0	2	0	1	1.857e-3	4	NC	1	NC	1
300			min	-.004	3	-.005	3	-.011	4	0	1	NC	1	7161.838	4
301		18	max	.001	2	0	2	0	1	1.888e-3	4	NC	1	NC	1
302			min	-.002	3	-.003	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	1.919e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-4.93e-4	4	NC	1	NC	1
307		2	max	.001	3	0	2	.009	4	0	1	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	-6.236e-5	5	NC	1	9996.233	4
309		3	max	.003	3	0	2	.017	4	3.691e-4	4	NC	1	NC	1
310			min	-.003	2	-.006	3	0	1	0	1	NC	1	5250.583	4
311		4	max	.004	3	-.001	15	.024	4	8.002e-4	4	NC	1	NC	1
312			min	-.004	2	-.008	3	0	1	0	1	NC	1	3671.545	4
313		5	max	.006	3	-.002	15	.031	4	1.231e-3	4	NC	1	NC	1
314			min	-.005	2	-.01	3	0	1	0	1	NC	1	2881.931	4
315		6	max	.007	3	-.002	15	.037	4	1.662e-3	4	NC	1	NC	1
316			min	-.007	2	-.012	3	0	1	0	1	8819.79	3	2405.809	4
317		7	max	.008	3	-.003	15	.043	4	2.093e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.008	2	-.014	3	0	1	0	1	7881.887	3	2084.119	4
319	8	max	.01	3	-.003	15	.049	4	2.524e-3	4	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7308.053	4	1848.378	4
321	9	max	.011	3	-.003	15	.054	4	2.955e-3	4	NC	1	NC	1
322		min	-.011	2	-.016	3	0	1	0	1	6849.686	4	1664.149	4
323	10	max	.012	3	-.003	15	.059	4	3.386e-3	4	NC	1	NC	1
324		min	-.012	2	-.017	3	0	1	0	1	6638.342	4	1512.25	4
325	11	max	.014	3	-.003	15	.065	4	3.818e-3	4	NC	1	NC	1
326		min	-.013	2	-.017	3	0	1	0	1	6643.144	4	1381.274	4
327	12	max	.015	3	-.003	15	.071	4	4.249e-3	4	NC	1	NC	1
328		min	-.015	2	-.017	3	0	1	0	1	6869.406	4	1264.216	4
329	13	max	.017	3	-.003	15	.078	4	4.68e-3	4	NC	1	NC	1
330		min	-.016	2	-.016	3	0	1	0	1	7362.209	4	1156.782	4
331	14	max	.018	3	-.003	15	.085	4	5.111e-3	4	NC	1	NC	1
332		min	-.018	2	-.015	3	0	1	0	1	8229.28	4	1056.452	4
333	15	max	.019	3	-.002	15	.093	4	5.542e-3	4	NC	1	NC	1
334		min	-.019	2	-.014	3	0	1	0	1	9707.518	4	961.897	4
335	16	max	.021	3	-.002	15	.103	4	5.973e-3	4	NC	1	NC	1
336		min	-.02	2	-.013	3	0	1	0	1	NC	1	872.573	4
337	17	max	.022	3	0	2	.114	4	6.404e-3	4	NC	1	NC	1
338		min	-.022	2	-.011	3	0	1	0	1	NC	1	788.414	4
339	18	max	.023	3	0	2	.127	4	6.835e-3	4	NC	1	NC	1
340		min	-.023	2	-.009	3	0	1	0	1	NC	1	709.595	4
341	19	max	.025	3	.002	2	.141	4	7.266e-3	4	NC	1	NC	1
342		min	-.024	2	-.008	3	0	1	0	1	NC	1	636.357	4
343	M8	1	max	.004	2	.024	2	0	1.003e-3	4	NC	1	NC	1
344		min	0	15	-.026	3	-.141	4	0	1	NC	1	175.705	4
345	2	max	.004	2	.023	2	0	1	1.003e-3	4	NC	1	NC	1
346		min	0	15	-.024	3	-.13	4	0	1	NC	1	190.487	4
347	3	max	.004	2	.021	2	0	1	1.003e-3	4	NC	1	NC	1
348		min	0	15	-.023	3	-.119	4	0	1	NC	1	208.115	4
349	4	max	.004	2	.02	2	0	1	1.003e-3	4	NC	1	NC	1
350		min	0	15	-.022	3	-.108	4	0	1	NC	1	229.322	4
351	5	max	.003	2	.019	2	0	1	1.003e-3	4	NC	1	NC	1
352		min	0	15	-.02	3	-.097	4	0	1	NC	1	255.105	4
353	6	max	.003	2	.017	2	0	1	1.003e-3	4	NC	1	NC	1
354		min	0	15	-.019	3	-.086	4	0	1	NC	1	286.845	4
355	7	max	.003	2	.016	2	0	1	1.003e-3	4	NC	1	NC	1
356		min	0	15	-.017	3	-.076	4	0	1	NC	1	326.504	4
357	8	max	.003	2	.015	2	0	1	1.003e-3	4	NC	1	NC	1
358		min	0	15	-.016	3	-.066	4	0	1	NC	1	376.942	4
359	9	max	.002	2	.013	2	0	1	1.003e-3	4	NC	1	NC	1
360		min	0	15	-.014	3	-.056	4	0	1	NC	1	442.464	4
361	10	max	.002	2	.012	2	0	1	1.003e-3	4	NC	1	NC	1
362		min	0	15	-.013	3	-.047	4	0	1	NC	1	529.797	4
363	11	max	.002	2	.011	2	0	1	1.003e-3	4	NC	1	NC	1
364		min	0	15	-.012	3	-.038	4	0	1	NC	1	649.94	4
365	12	max	.002	2	.009	2	0	1	1.003e-3	4	NC	1	NC	1
366		min	0	15	-.01	3	-.03	4	0	1	NC	1	821.911	4
367	13	max	.001	2	.008	2	0	1	1.003e-3	4	NC	1	NC	1
368		min	0	15	-.009	3	-.023	4	0	1	NC	1	1080.987	4
369	14	max	.001	2	.007	2	0	1	1.003e-3	4	NC	1	NC	1
370		min	0	15	-.007	3	-.017	4	0	1	NC	1	1498.889	4
371	15	max	0	2	.005	2	0	1	1.003e-3	4	NC	1	NC	1
372		min	0	15	-.006	3	-.011	4	0	1	NC	1	2240.815	4
373	16	max	0	2	.004	2	0	1	1.003e-3	4	NC	1	NC	1
374		min	0	15	-.004	3	-.007	4	0	1	NC	1	3762.963	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.003	2	0	1	1.003e-3	4	NC	1	NC	1
376			min	0	15	-.003	3	-.003	4	0	1	NC	1	7754.344	4
377		18	max	0	2	.001	2	0	1	1.003e-3	4	NC	1	NC	1
378			min	0	15	-.001	3	0	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	1.003e-3	4	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.01	2	0	10	1.355e-3	4	NC	1	NC	1
382			min	-.01	3	-.016	3	-.422	4	1.057e-5	10	7453.502	2	182.318	4
383		2	max	.007	2	.009	2	0	10	1.386e-3	4	NC	1	NC	1
384			min	-.01	3	-.015	3	-.389	4	1.002e-5	10	8642.116	2	198.117	4
385		3	max	.006	2	.008	2	0	10	1.416e-3	4	NC	1	NC	1
386			min	-.009	3	-.015	3	-.355	4	9.477e-6	10	NC	1	216.811	4
387		4	max	.006	2	.006	2	0	10	1.447e-3	4	NC	1	NC	1
388			min	-.009	3	-.014	3	-.322	4	8.93e-6	10	NC	1	239.146	4
389		5	max	.005	2	.005	2	0	10	1.477e-3	4	NC	1	NC	1
390			min	-.008	3	-.014	3	-.289	4	8.383e-6	10	NC	1	266.131	4
391		6	max	.005	2	.004	2	0	10	1.507e-3	4	NC	1	NC	1
392			min	-.007	3	-.013	3	-.257	4	7.836e-6	10	NC	1	299.156	4
393		7	max	.005	2	.002	2	0	10	1.538e-3	4	NC	1	NC	1
394			min	-.007	3	-.012	3	-.226	4	7.289e-6	10	NC	1	340.188	4
395		8	max	.004	2	.001	2	0	10	1.568e-3	4	NC	1	NC	1
396			min	-.006	3	-.012	3	-.196	4	6.742e-6	10	NC	1	392.074	4
397		9	max	.004	2	0	2	0	10	1.598e-3	4	NC	1	NC	1
398			min	-.006	3	-.011	3	-.168	4	6.195e-6	10	NC	1	459.074	4
399		10	max	.004	2	0	2	0	10	1.629e-3	4	NC	1	NC	1
400			min	-.005	3	-.01	3	-.141	4	5.648e-6	10	NC	1	547.796	4
401		11	max	.003	2	0	2	0	10	1.659e-3	4	NC	1	NC	1
402			min	-.005	3	-.009	3	-.115	4	5.101e-6	10	NC	1	668.95	4
403		12	max	.003	2	-.001	2	0	10	1.69e-3	4	NC	1	NC	1
404			min	-.004	3	-.008	3	-.092	4	4.554e-6	10	NC	1	840.864	4
405		13	max	.002	2	-.002	15	0	10	1.72e-3	4	NC	1	NC	1
406			min	-.003	3	-.007	3	-.07	4	4.007e-6	10	NC	1	1097.1	4
407		14	max	.002	2	-.002	15	0	10	1.75e-3	4	NC	1	NC	1
408			min	-.003	3	-.006	3	-.051	4	3.459e-6	10	NC	1	1504.778	4
409		15	max	.002	2	-.001	15	0	10	1.781e-3	4	NC	1	NC	1
410			min	-.002	3	-.005	3	-.035	4	2.912e-6	10	NC	1	2215.154	4
411		16	max	.001	2	-.001	15	0	10	1.811e-3	4	NC	1	NC	1
412			min	-.002	3	-.004	4	-.021	4	2.365e-6	10	NC	1	3633.241	4
413		17	max	0	2	0	15	0	10	1.841e-3	4	NC	1	NC	1
414			min	-.001	3	-.003	4	-.011	4	1.818e-6	10	NC	1	7188.842	4
415		18	max	0	2	0	15	0	10	1.872e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.004	4	1.271e-6	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	1.902e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	7.243e-7	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-1.592e-7	10	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.883e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.009	4	-1.249e-6	12	NC	1	NC	1
422			min	0	2	-.002	4	0	10	-5.303e-5	4	NC	1	NC	1
423		3	max	0	3	-.001	15	.017	4	3.835e-4	5	NC	1	NC	1
424			min	0	2	-.004	4	0	10	-2.117e-5	1	NC	1	5300.092	4
425		4	max	.001	3	-.002	15	.024	4	8.177e-4	5	NC	1	NC	1
426			min	-.001	2	-.006	4	0	10	-3.086e-5	1	NC	1	3706.207	4
427		5	max	.002	3	-.002	15	.031	4	1.253e-3	4	NC	1	NC	1
428			min	-.002	2	-.008	4	0	10	-4.056e-5	1	NC	1	2908.632	4
429		6	max	.002	3	-.003	15	.037	4	1.688e-3	4	NC	1	NC	1
430			min	-.002	2	-.01	4	0	10	-5.025e-5	1	9135.012	4	2427.159	4
431		7	max	.003	3	-.003	15	.043	4	2.123e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.012	4	0	1	-5.995e-5	1	7906.925	4	2101.3	4
433		8	max	.003	3	-.003	15	.048	4	2.558e-3	4	NC	2	NC	1
434			min	-.003	2	-.013	4	0	1	-6.964e-5	1	7151.475	4	1861.97	4
435		9	max	.004	3	-.003	15	.054	4	2.994e-3	4	NC	5	NC	1
436			min	-.003	2	-.014	4	0	1	-7.933e-5	1	6711.525	4	1674.451	4
437		10	max	.004	3	-.004	15	.059	4	3.429e-3	4	NC	5	NC	1
438			min	-.004	2	-.014	4	0	1	-8.903e-5	1	6511.508	4	1519.442	4
439		11	max	.005	3	-.004	15	.065	4	3.864e-3	4	NC	5	NC	1
440			min	-.004	2	-.014	4	0	1	-9.872e-5	1	6522.23	4	1385.506	4
441		12	max	.005	3	-.004	15	.071	4	4.299e-3	4	NC	5	NC	1
442			min	-.005	2	-.014	4	0	1	-1.084e-4	1	6749.656	4	1265.662	4
443		13	max	.005	3	-.003	15	.078	4	4.734e-3	4	NC	2	NC	1
444			min	-.005	2	-.013	4	-.001	1	-1.181e-4	1	7238.656	4	1155.673	4
445		14	max	.006	3	-.003	15	.085	4	5.17e-3	4	NC	1	NC	1
446			min	-.005	2	-.012	4	-.001	1	-1.278e-4	1	8095.651	4	1053.085	4
447		15	max	.006	3	-.003	15	.094	4	5.605e-3	4	NC	1	NC	1
448			min	-.006	2	-.01	4	-.002	1	-1.375e-4	1	9554.212	4	956.632	4
449		16	max	.007	3	-.002	15	.104	4	6.04e-3	4	NC	1	NC	1
450			min	-.006	2	-.008	4	-.002	1	-1.472e-4	1	NC	1	865.808	4
451		17	max	.007	3	-.002	15	.115	4	6.475e-3	4	NC	1	NC	1
452			min	-.007	2	-.006	4	-.003	1	-1.569e-4	1	NC	1	780.562	4
453		18	max	.008	3	-.001	10	.128	4	6.911e-3	4	NC	1	NC	1
454			min	-.007	2	-.004	3	-.003	1	-1.666e-4	1	NC	1	701.051	4
455		19	max	.008	3	0	2	.143	4	7.346e-3	4	NC	1	NC	1
456			min	-.007	2	-.003	3	-.004	1	-1.763e-4	1	NC	1	627.477	4
457	M12	1	max	.002	1	.007	2	.004	1	1.08e-3	5	NC	1	NC	2
458			min	0	12	-.009	3	-.143	4	-7.021e-5	1	NC	1	173.253	4
459		2	max	.002	1	.007	2	.003	1	1.08e-3	5	NC	1	NC	2
460			min	0	12	-.008	3	-.132	4	-7.021e-5	1	NC	1	187.816	4
461		3	max	.002	1	.006	2	.003	1	1.08e-3	5	NC	1	NC	2
462			min	0	12	-.008	3	-.121	4	-7.021e-5	1	NC	1	205.183	4
463		4	max	.001	1	.006	2	.003	1	1.08e-3	5	NC	1	NC	2
464			min	0	12	-.007	3	-.11	4	-7.021e-5	1	NC	1	226.078	4
465		5	max	.001	1	.006	2	.003	1	1.08e-3	5	NC	1	NC	2
466			min	0	12	-.007	3	-.099	4	-7.021e-5	1	NC	1	251.482	4
467		6	max	.001	1	.005	2	.002	1	1.08e-3	5	NC	1	NC	1
468			min	0	12	-.006	3	-.088	4	-7.021e-5	1	NC	1	282.756	4
469		7	max	.001	1	.005	2	.002	1	1.08e-3	5	NC	1	NC	1
470			min	0	12	-.006	3	-.077	4	-7.021e-5	1	NC	1	321.834	4
471		8	max	.001	1	.004	2	.002	1	1.08e-3	5	NC	1	NC	1
472			min	0	12	-.005	3	-.067	4	-7.021e-5	1	NC	1	371.533	4
473		9	max	0	1	.004	2	.001	1	1.08e-3	5	NC	1	NC	1
474			min	0	12	-.005	3	-.057	4	-7.021e-5	1	NC	1	436.095	4
475		10	max	0	1	.004	2	.001	1	1.08e-3	5	NC	1	NC	1
476			min	0	12	-.004	3	-.048	4	-7.021e-5	1	NC	1	522.148	4
477		11	max	0	1	.003	2	.001	1	1.08e-3	5	NC	1	NC	1
478			min	0	12	-.004	3	-.039	4	-7.021e-5	1	NC	1	640.529	4
479		12	max	0	1	.003	2	0	1	1.08e-3	5	NC	1	NC	1
480			min	0	12	-.003	3	-.031	4	-7.021e-5	1	NC	1	809.977	4
481		13	max	0	1	.002	2	0	1	1.08e-3	5	NC	1	NC	1
482			min	0	12	-.003	3	-.023	4	-7.021e-5	1	NC	1	1065.249	4
483		14	max	0	1	.002	2	0	1	1.08e-3	5	NC	1	NC	1
484			min	0	12	-.002	3	-.017	4	-7.021e-5	1	NC	1	1477.011	4
485		15	max	0	1	.002	2	0	1	1.08e-3	5	NC	1	NC	1
486			min	0	12	-.002	3	-.011	4	-7.021e-5	1	NC	1	2208.023	4
487		16	max	0	1	.001	2	0	1	1.08e-3	5	NC	1	NC	1
488			min	0	12	-.001	3	-.007	4	-7.021e-5	1	NC	1	3707.745	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	1.08e-3	5	NC	1	NC	1
490		min	0	12	0	3	-.003	4	-7.021e-5	1	NC	1	7640.196	4
491	18	max	0	1	0	2	0	1	1.08e-3	5	NC	1	NC	1
492		min	0	12	0	3	0	4	-7.021e-5	1	NC	1	NC	1
493	19	max	0	1	0	1	0	1	1.08e-3	5	NC	1	NC	1
494		min	0	1	0	1	0	1	-7.021e-5	1	NC	1	NC	1
495	M1	1	max	.01	.112	2	.445	4	5.338e-3	2	NC	1	NC	1
496		min	-.006	2	-.031	3	0	10	-1.324e-2	3	NC	1	NC	1
497	2	max	.01	3	.052	2	.433	4	4.212e-3	4	NC	4	NC	1
498		min	-.006	2	-.011	3	-.003	1	-6.554e-3	3	1909.374	2	NC	1
499	3	max	.01	3	.017	3	.419	4	7.61e-3	4	NC	5	NC	1
500		min	-.006	2	-.012	2	-.004	1	-9.236e-5	3	926.228	2	8648.049	5
501	4	max	.01	3	.058	3	.406	4	6.505e-3	4	NC	5	NC	1
502		min	-.006	2	-.083	2	-.004	1	-3.15e-3	3	590.258	2	6316.468	5
503	5	max	.01	3	.108	3	.391	4	5.559e-3	2	NC	5	NC	1
504		min	-.006	2	-.156	2	-.002	1	-6.208e-3	3	429.476	2	5147.722	5
505	6	max	.009	3	.161	3	.376	4	8.327e-3	2	NC	5	NC	1
506		min	-.006	2	-.226	2	-.001	1	-9.266e-3	3	340.404	2	4429.939	5
507	7	max	.009	3	.211	3	.361	4	1.109e-2	2	NC	15	NC	1
508		min	-.006	2	-.289	2	0	3	-1.232e-2	3	287.572	2	3901.059	4
509	8	max	.009	3	.252	3	.346	4	1.386e-2	2	NC	15	NC	1
510		min	-.006	2	-.338	2	0	12	-1.538e-2	3	256.21	2	3483.329	4
511	9	max	.009	3	.279	3	.33	4	1.569e-2	2	NC	15	NC	1
512		min	-.006	2	-.369	2	0	1	-1.581e-2	3	239.839	2	3180.59	4
513	10	max	.009	3	.288	3	.313	4	1.688e-2	2	NC	15	NC	1
514		min	-.006	2	-.379	2	0	10	-1.448e-2	3	235.071	2	3059.145	4
515	11	max	.008	3	.281	3	.294	4	1.807e-2	2	NC	15	NC	1
516		min	-.006	2	-.368	2	0	12	-1.316e-2	3	240.805	2	3066.974	4
517	12	max	.008	3	.257	3	.274	4	1.742e-2	2	NC	15	NC	1
518		min	-.005	2	-.336	2	0	1	-1.145e-2	3	259.09	2	3198.807	4
519	13	max	.008	3	.219	3	.25	4	1.396e-2	2	NC	15	NC	1
520		min	-.005	2	-.284	2	0	1	-9.167e-3	3	294.457	2	3668.045	4
521	14	max	.008	3	.171	3	.224	4	1.051e-2	2	NC	5	NC	1
522		min	-.005	2	-.218	2	0	12	-6.88e-3	3	354.897	2	4711.111	4
523	15	max	.008	3	.117	3	.198	4	7.062e-3	2	NC	5	NC	1
524		min	-.005	2	-.146	2	0	12	-4.593e-3	3	458.865	2	7006.364	4
525	16	max	.007	3	.061	3	.171	4	6.211e-3	4	NC	5	NC	1
526		min	-.005	2	-.074	2	0	12	-2.307e-3	3	651.208	2	NC	1
527	17	max	.007	3	.006	3	.146	4	7.25e-3	4	NC	5	NC	1
528		min	-.005	2	-.007	2	0	12	-1.973e-5	3	1061.986	2	NC	1
529	18	max	.007	3	.049	2	.125	4	4.809e-3	2	NC	4	NC	1
530		min	-.005	2	-.043	3	0	12	-1.948e-3	3	2251.118	2	NC	1
531	19	max	.007	3	.099	2	.106	4	9.647e-3	2	NC	1	NC	1
532		min	-.005	2	-.09	3	0	1	-3.974e-3	3	NC	1	NC	1
533	M5	1	max	.03	.197	2	.445	4	0	1	NC	1	NC	1
534		min	-.022	2	-.003	3	0	1	-9.733e-6	4	NC	1	NC	1
535	2	max	.03	3	.087	2	.435	4	3.904e-3	4	NC	4	NC	1
536		min	-.022	2	.002	15	0	1	0	1	1055.709	2	NC	1
537	3	max	.03	3	.051	3	.423	4	7.697e-3	4	NC	5	NC	1
538		min	-.022	2	-.037	2	0	1	0	1	495.631	2	7144.701	4
539	4	max	.03	3	.135	3	.409	4	6.27e-3	4	NC	5	NC	1
540		min	-.021	2	-.186	2	0	1	0	1	302.509	2	5570.014	4
541	5	max	.029	3	.25	3	.393	4	4.843e-3	4	NC	15	NC	1
542		min	-.021	2	-.348	2	0	1	0	1	212.444	2	4823.842	4
543	6	max	.028	3	.38	3	.377	4	3.416e-3	4	8926.96	15	NC	1
544		min	-.021	2	-.509	2	0	1	0	1	163.941	2	4358.447	4
545	7	max	.028	3	.505	3	.361	4	1.989e-3	4	7369.724	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.02	2	-.655	2	0	1	0	1	135.84	2	3964.988	4
547	8	max	.027	3	.611	3	.345	4	5.624e-4	4	6466.872	15	NC	1
548		min	-.02	2	-.772	2	0	1	0	1	119.463	2	3546.025	4
549	9	max	.026	3	.678	3	.33	4	0	1	6004.867	15	NC	1
550		min	-.02	2	-.847	2	0	1	-7.345e-6	5	111.05	2	3170.659	4
551	10	max	.026	3	.701	3	.313	4	0	1	5865.911	15	NC	1
552		min	-.019	2	-.872	2	0	1	-7.143e-6	5	108.606	2	3080.98	4
553	11	max	.025	3	.682	3	.294	4	0	1	6005.453	15	NC	1
554		min	-.019	2	-.846	2	0	1	-6.942e-6	5	111.545	2	3106.398	4
555	12	max	.025	3	.623	3	.274	4	5.096e-4	4	6468.214	15	NC	1
556		min	-.019	2	-.768	2	0	1	0	1	121.116	2	3141.805	4
557	13	max	.024	3	.527	3	.251	4	1.803e-3	4	7372.324	15	NC	1
558		min	-.018	2	-.642	2	0	1	0	1	140.196	2	3597.559	4
559	14	max	.023	3	.407	3	.224	4	3.096e-3	4	8931.839	15	NC	1
560		min	-.018	2	-.487	2	0	1	0	1	173.905	2	4857.736	4
561	15	max	.023	3	.274	3	.195	4	4.388e-3	4	NC	15	NC	1
562		min	-.018	2	-.319	2	0	1	0	1	234.541	2	8339.319	4
563	16	max	.022	3	.141	3	.167	4	5.681e-3	4	NC	5	NC	1
564		min	-.017	2	-.158	2	0	1	0	1	353.599	2	NC	1
565	17	max	.021	3	.017	3	.142	4	6.974e-3	4	NC	5	NC	1
566		min	-.017	2	-.02	2	0	1	0	1	625.428	2	NC	1
567	18	max	.021	3	.079	2	.121	4	3.539e-3	4	NC	4	NC	1
568		min	-.017	2	-.086	3	0	1	0	1	1388.956	3	NC	1
569	19	max	.021	3	.158	2	.107	4	0	1	NC	1	NC	1
570		min	-.017	2	-.178	3	0	1	-6.024e-6	4	NC	1	NC	1
571	M9	1	max	.01	.112	2	.445	4	1.324e-2	3	NC	1	NC	1
572		min	-.006	2	-.031	3	0	1	-5.338e-3	2	NC	1	NC	1
573	2	max	.01	3	.052	2	.434	4	6.554e-3	3	NC	4	NC	1
574		min	-.006	2	-.011	3	0	10	-2.618e-3	2	1909.374	2	NC	1
575	3	max	.01	3	.017	3	.422	4	7.674e-3	4	NC	5	NC	1
576		min	-.006	2	-.012	2	0	10	-2.566e-5	10	926.228	2	7562.551	4
577	4	max	.01	3	.058	3	.408	4	6.101e-3	5	NC	5	NC	1
578		min	-.006	2	-.083	2	0	10	-2.791e-3	2	590.258	2	5751.636	4
579	5	max	.01	3	.108	3	.393	4	6.208e-3	3	NC	5	NC	1
580		min	-.006	2	-.156	2	0	10	-5.559e-3	2	429.476	2	4867.871	4
581	6	max	.009	3	.161	3	.377	4	9.266e-3	3	NC	5	NC	1
582		min	-.006	2	-.226	2	0	10	-8.327e-3	2	340.404	2	4321.736	4
583	7	max	.009	3	.211	3	.361	4	1.232e-2	3	NC	15	NC	1
584		min	-.006	2	-.289	2	0	1	-1.109e-2	2	287.572	2	3901.493	4
585	8	max	.009	3	.252	3	.346	4	1.538e-2	3	NC	15	NC	1
586		min	-.006	2	-.338	2	0	1	-1.386e-2	2	256.21	2	3510.016	4
587	9	max	.009	3	.279	3	.33	4	1.581e-2	3	NC	15	NC	1
588		min	-.006	2	-.369	2	0	10	-1.569e-2	2	239.839	2	3172.468	4
589	10	max	.009	3	.288	3	.313	4	1.448e-2	3	NC	15	NC	1
590		min	-.006	2	-.379	2	0	1	-1.688e-2	2	235.071	2	3060.243	4
591	11	max	.008	3	.281	3	.294	4	1.316e-2	3	NC	15	NC	1
592		min	-.006	2	-.368	2	0	1	-1.807e-2	2	240.805	2	3076.263	4
593	12	max	.008	3	.257	3	.274	4	1.145e-2	3	NC	15	NC	1
594		min	-.005	2	-.336	2	0	10	-1.742e-2	2	259.09	2	3176.736	4
595	13	max	.008	3	.219	3	.25	4	9.167e-3	3	NC	15	NC	1
596		min	-.005	2	-.284	2	0	10	-1.396e-2	2	294.457	2	3664.159	4
597	14	max	.008	3	.171	3	.224	4	6.88e-3	3	NC	5	NC	1
598		min	-.005	2	-.218	2	0	1	-1.051e-2	2	354.897	2	4828.072	5
599	15	max	.008	3	.117	3	.196	4	4.593e-3	3	NC	5	NC	1
600		min	-.005	2	-.146	2	-.002	1	-7.062e-3	2	458.865	2	7541.108	5
601	16	max	.007	3	.061	3	.169	4	5.698e-3	5	NC	5	NC	1
602		min	-.005	2	-.074	2	-.003	1	-3.611e-3	2	651.208	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.007	3	.006	3	.144	4	7.093e-3	4	NC	5	NC	1
604		min	-.005	2	-.007	2	-.004	1	-2.619e-4	1	1061.986	2	NC	1
605	18	max	.007	3	.049	2	.123	4	3.512e-3	5	NC	4	NC	1
606		min	-.005	2	-.043	3	-.003	1	-4.809e-3	2	2251.118	2	NC	1
607	19	max	.007	3	.099	2	.106	4	3.974e-3	3	NC	1	NC	1
608		min	-.005	2	-.09	3	0	12	-9.647e-3	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

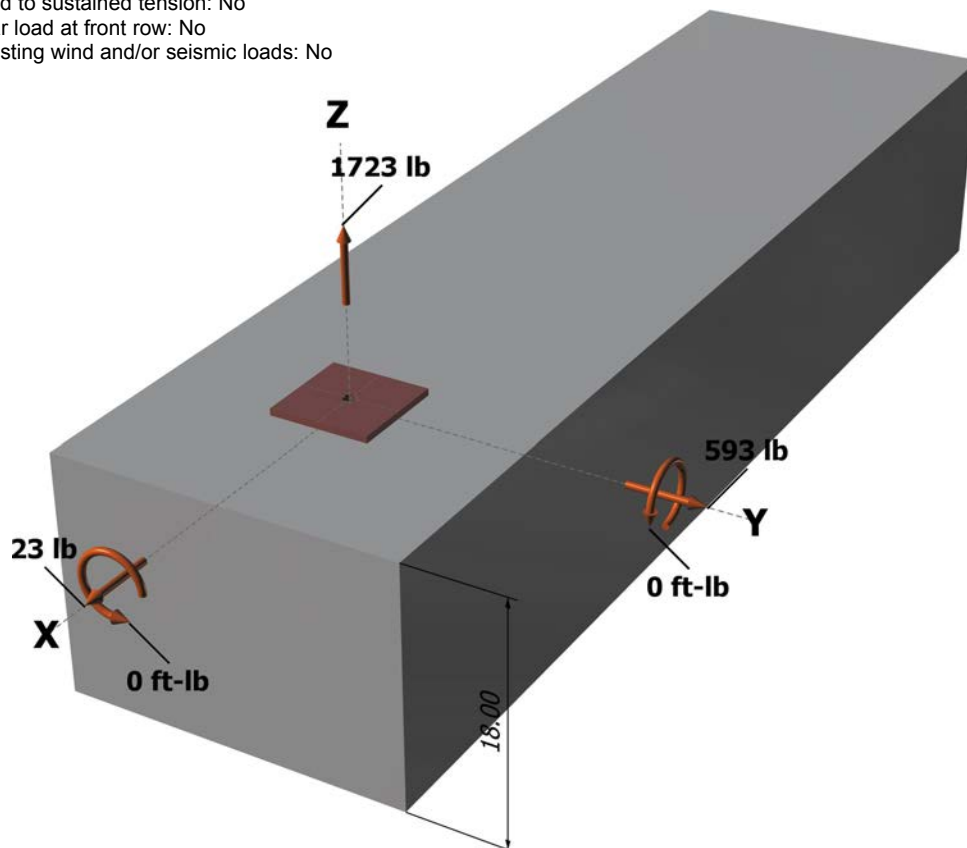
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Software  
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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

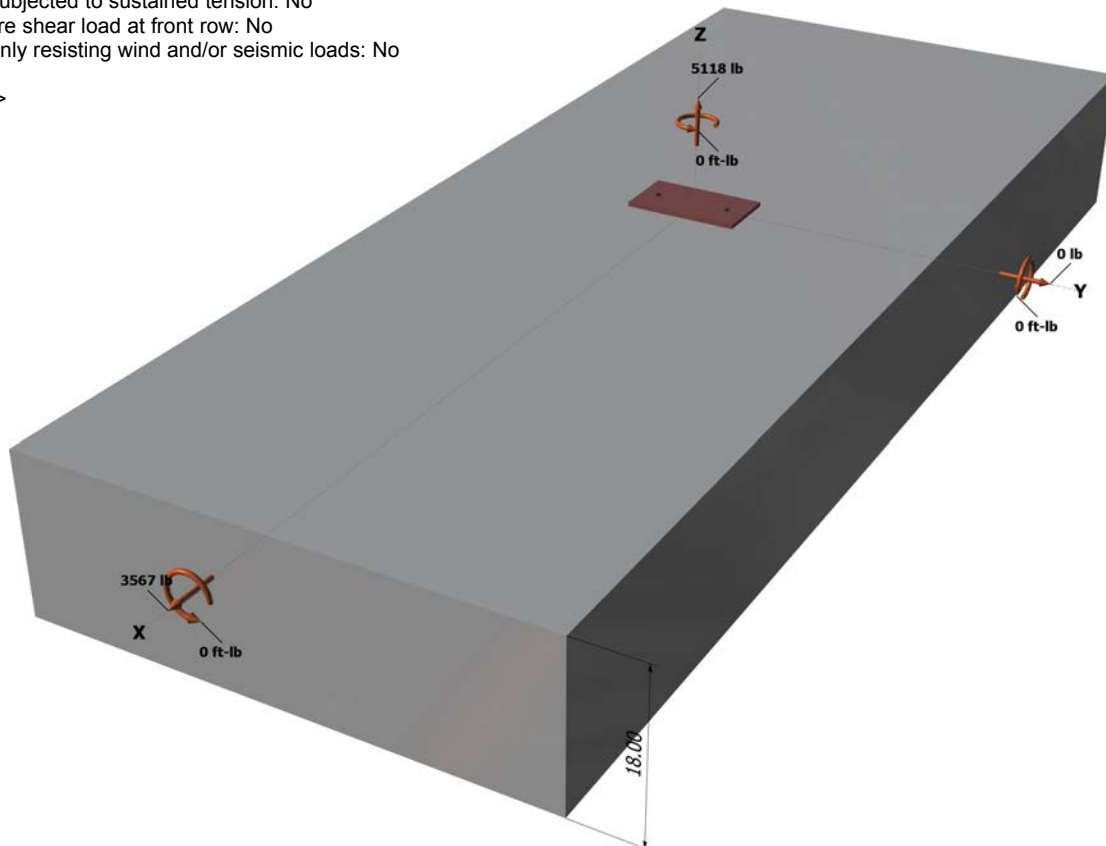
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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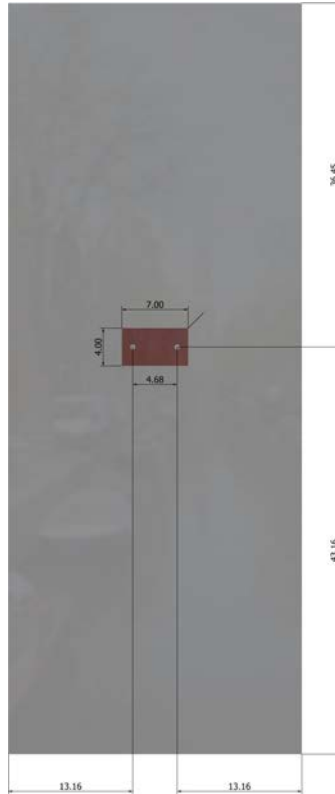




Anchor Designer™  
Software  
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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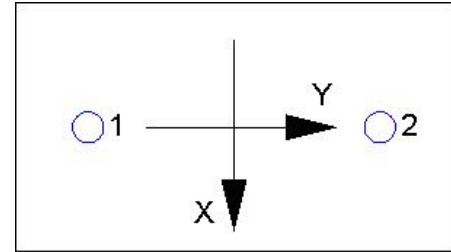
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5118  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
<b>Adhesive</b>	<b>5118</b>	<b>8093</b>	<b>0.63</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1784</b>	<b>3156</b>	<b>0.57</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.