



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	30° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	150 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 35.33 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

C_{f+} TOP =	1.150	(Pressure)
C_{f+} BOTTOM =	1.850	
C_{f-} TOP, OUTER PURLIN =	-2.600	
C_{f-} TOP, INNER PURLIN =	-2.000	(Suction)
C_{f-} BOTTOM =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	84 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.559 k-ft
M_z =	0.093 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	64%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

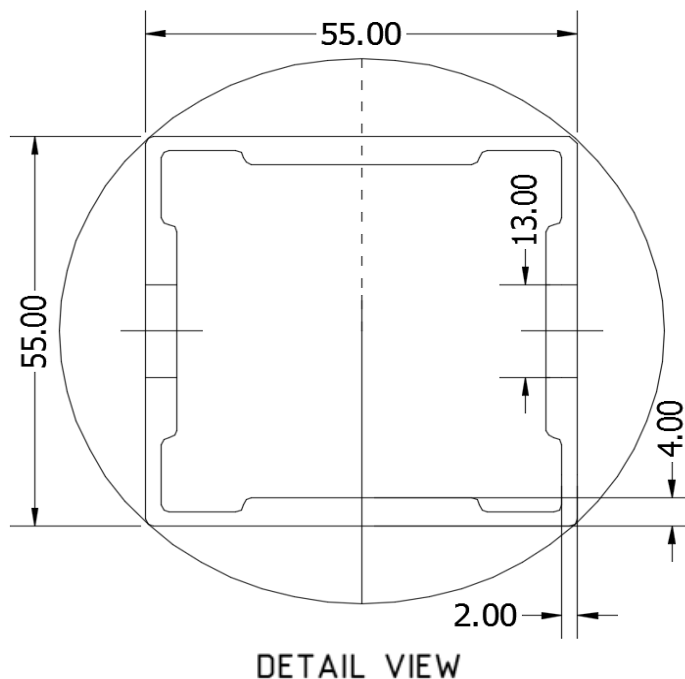
Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.272 k-ft
M_z =	0.000 k-ft
P_n =	-1.029 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	97%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

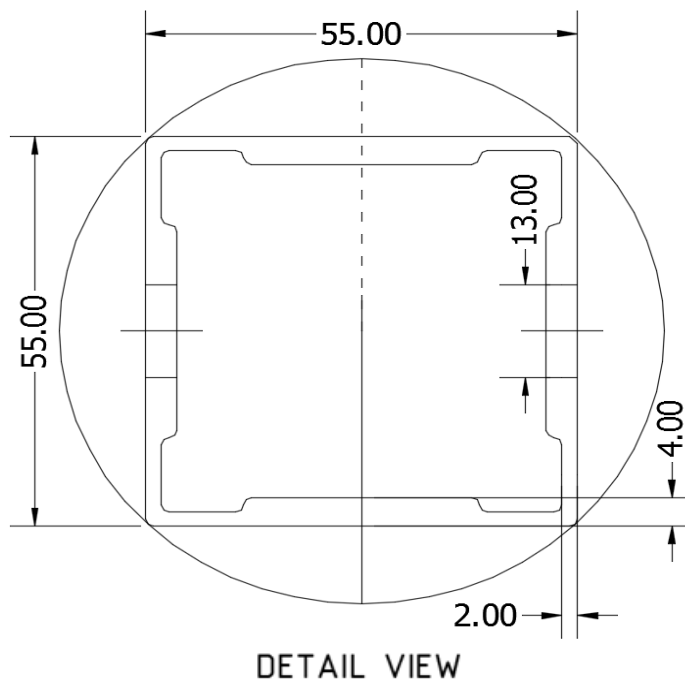
Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	2.441 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.012 k-ft
M_z =	0.000 k-ft
P_n =	2.734 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	46%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.012 k-ft
M_z =	0.000 k-ft
P_n =	3.248 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	38%



5. FOUNDATION DESIGN CALCULATIONS

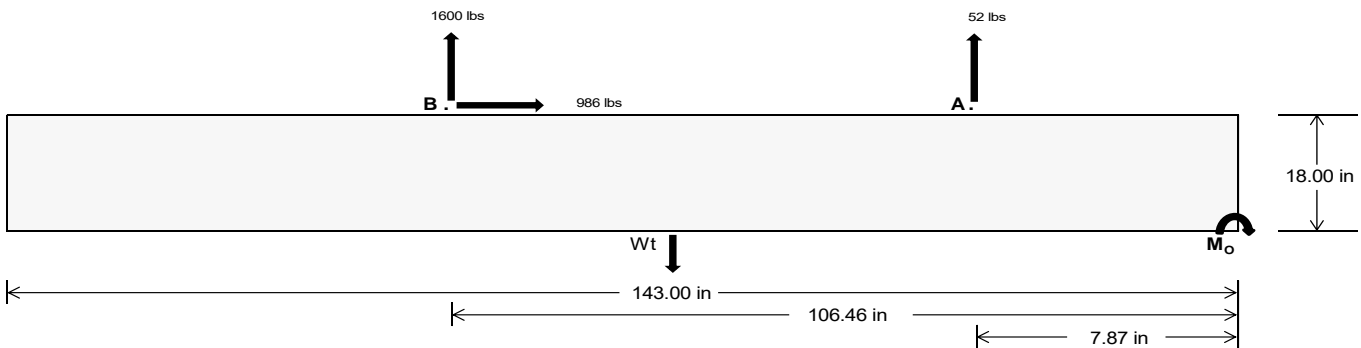
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		242.11	6946.39 k
Compressive Load =		3173.91	5076.80 k
Lateral Load =		11.13	4270.60 k
Moment (Weak Axis) =		0.02	0.00 k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 188480.4$ in-lbs
Resisting Force Required = 2636.09 lbs
S.F. = 1.67
Weight Required = 4393.48 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 985.73 lbs
Friction = 0.4
Weight Required = 2464.34 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 985.73 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$ 7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	983 lbs	983 lbs	983 lbs	983 lbs	1292 lbs	1292 lbs	1292 lbs	1292 lbs	1601 lbs	1601 lbs	1601 lbs	1601 lbs	-104 lbs	-104 lbs	-104 lbs	-104 lbs
F_B	955 lbs	955 lbs	955 lbs	955 lbs	2173 lbs	2173 lbs	2173 lbs	2173 lbs	2245 lbs	2245 lbs	2245 lbs	2245 lbs	-3200 lbs	-3200 lbs	-3200 lbs	-3200 lbs
F_V	120 lbs	120 lbs	120 lbs	120 lbs	1769 lbs	1769 lbs	1769 lbs	1769 lbs	1405 lbs	1405 lbs	1405 lbs	1405 lbs	-1971 lbs	-1971 lbs	-1971 lbs	-1971 lbs
P_{total}	9498 lbs	9714 lbs	9930 lbs	10146 lbs	11025 lbs	11241 lbs	11457 lbs	11673 lbs	11405 lbs	11621 lbs	11837 lbs	12053 lbs	1232 lbs	1362 lbs	1491 lbs	1621 lbs
M	2608 lbs-ft	2608 lbs-ft	2608 lbs-ft	2608 lbs-ft	3174 lbs-ft	3174 lbs-ft	3174 lbs-ft	3174 lbs-ft	4057 lbs-ft	4057 lbs-ft	4057 lbs-ft	4057 lbs-ft	5815 lbs-ft	5815 lbs-ft	5815 lbs-ft	5815 lbs-ft
e	0.27 ft	0.27 ft	0.26 ft	0.26 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.36 ft	0.35 ft	0.34 ft	0.34 ft	4.72 ft	4.27 ft	3.90 ft	3.59 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	235.5 psf	235.0 psf	234.5 psf	234.1 psf	271.2 psf	269.7 psf	268.3 psf	267.0 psf	269.4 psf	267.9 psf	266.6 psf	265.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	311.0 psf	308.5 psf	306.0 psf	303.7 psf	363.2 psf	359.1 psf	355.3 psf	351.7 psf	386.9 psf	382.2 psf	377.8 psf	373.5 psf	227.3 psf	179.3 psf	156.6 psf	143.9 psf

Maximum Bearing Pressure = 387 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

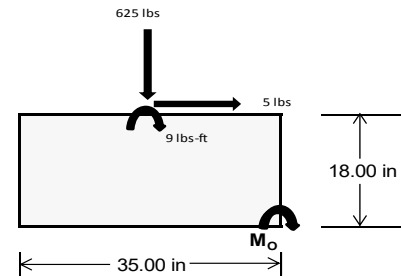
Overturning Check

$M_o = 895.1 \text{ ft-lbs}$
 Resisting Force Required = 613.79 lbs
 S.F. = 1.67
 Weight Required = 1022.98 lbs
 Minimum Width = **35 in**
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	222 lbs	514 lbs	222 lbs	625 lbs	1632 lbs	625 lbs	65 lbs	150 lbs	65 lbs
F_v	1 lbs	0 lbs	1 lbs	5 lbs	0 lbs	5 lbs	0 lbs	0 lbs	0 lbs
P_{total}	9581 lbs	7560 lbs	9581 lbs	9534 lbs	7560 lbs	9534 lbs	2802 lbs	7560 lbs	2802 lbs
M	5 lbs-ft	0 lbs-ft	5 lbs-ft	16 lbs-ft	0 lbs-ft	16 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	275.4 psf	217.5 psf	275.4 psf	273.4 psf	217.5 psf	273.4 psf	80.6 psf	217.5 psf	80.6 psf
f_{max}	276.0 psf	217.5 psf	276.0 psf	275.2 psf	217.5 psf	275.2 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 276 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 35in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

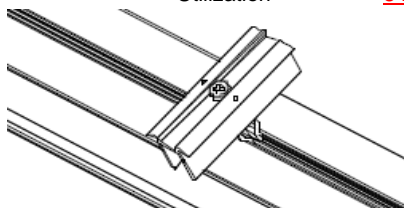
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

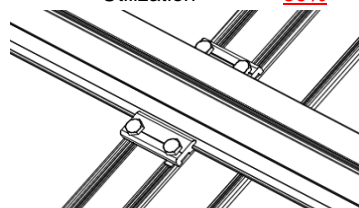
Fastening of Modules to Purlins

Maximum Uplifting Force =	1.020 k
Allowable Uplift =	1.214 k
Utilization =	<u>84%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.558 k
Allowable Uplift =	4.357 k
Utilization =	<u>59%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.441 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>33%</u>

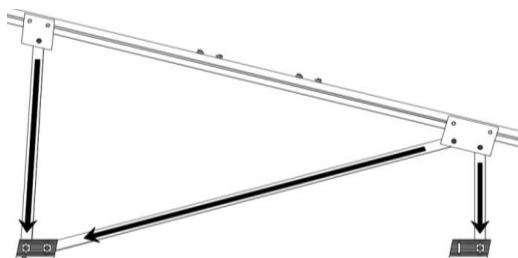
Rear Strut

Maximum Axial Load =	4.678 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>63%</u>

Diagonal Strut

Maximum Axial Load =	2.884 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

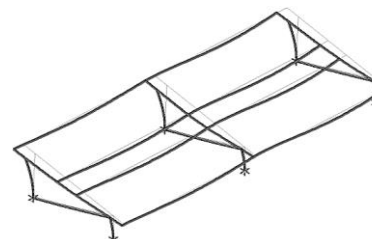
7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	60.93 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.219 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 84 \text{ in}$$

$$J = 0.432$$

$$232.383$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 28.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 84$$

$$J = 0.432$$

$$147.782$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

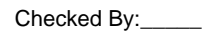
3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMax 72 Cell 2V 30° 150mph 30psf 7ft 7-10 NPS and 18





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	48.425	1	221.387	2	2.391	3	.013	2	-.004	15	.813	3
28			min	2.33	15	-366.406	3	-27.536	1	0	15	-.093	1	-.39	2
29		15	max	48.425	1	92.763	2	6.92	1	.013	2	-.004	12	1.008	3
30			min	2.33	15	-136.243	3	-.806	10	0	15	-.101	1	-.512	2
31		16	max	48.425	1	93.919	3	41.376	1	.013	2	-.001	12	1.025	3
32			min	2.33	15	-35.861	2	1.992	15	0	15	-.082	1	-.534	2
33		17	max	48.425	1	324.082	3	75.833	1	.013	2	.005	3	.862	3
34			min	2.33	15	-164.486	2	3.589	15	0	15	-.037	1	-.456	2
35		18	max	48.425	1	554.245	3	110.289	1	.013	2	.036	1	.521	3
36			min	2.33	15	-293.11	2	5.187	15	0	15	0	10	-.278	2
37		19	max	48.425	1	784.408	3	144.746	1	.013	2	.135	1	0	2
38			min	2.33	15	-421.734	2	6.784	15	0	15	.007	15	0	3
39	M14	1	max	33.001	1	510.465	2	-7.103	15	.015	3	.166	1	0	1
40			min	1.579	15	-656.389	3	-151.52	1	-.016	2	.008	15	0	3
41		2	max	33.001	1	381.841	2	-5.505	15	.015	3	.062	1	.442	3
42			min	1.579	15	-480.313	3	-117.064	1	-.016	2	.003	15	-.347	2
43		3	max	33.001	1	253.217	2	-3.908	15	.015	3	.007	3	.747	3
44			min	1.579	15	-304.238	3	-82.607	1	-.016	2	-.016	1	-.594	2
45		4	max	33.001	1	124.593	2	-2.311	15	.015	3	0	3	.915	3
46			min	1.579	15	-128.162	3	-48.151	1	-.016	2	-.067	1	-.741	2
47		5	max	33.001	1	47.913	3	.051	10	.015	3	-.003	12	.947	3
48			min	1.579	15	-7.853	1	-13.694	1	-.016	2	-.091	1	-.788	2
49		6	max	33.001	1	223.988	3	20.762	1	.015	3	-.004	15	.841	3
50			min	1.579	15	-132.655	2	-3.007	3	-.016	2	-.088	1	-.735	2
51		7	max	33.001	1	400.064	3	55.218	1	.015	3	-.003	15	.598	3
52			min	1.579	15	-261.28	2	-.571	3	-.016	2	-.058	1	-.581	2
53		8	max	33.001	1	576.139	3	89.675	1	.015	3	.004	2	.218	3
54			min	1.579	15	-389.904	2	1.528	12	-.016	2	-.009	3	-.328	2
55		9	max	33.001	1	752.215	3	124.131	1	.015	3	.081	1	.049	1
56			min	1.579	15	-518.528	2	3.151	12	-.016	2	-.007	3	-.298	3
57		10	max	33.001	1	647.152	2	-4.775	12	.016	2	.191	1	.478	2
58			min	1.579	15	-928.29	3	-158.588	1	-.015	3	-.002	3	-.952	3
59		11	max	33.001	1	518.528	2	-3.151	12	.016	2	.081	1	.049	1
60			min	1.579	15	-752.215	3	-124.131	1	-.015	3	-.007	3	-.298	3
61		12	max	33.001	1	389.904	2	-1.528	12	.016	2	.004	2	.218	3
62			min	1.579	15	-576.139	3	-89.675	1	-.015	3	-.009	3	-.328	2
63		13	max	33.001	1	261.28	2	.571	3	.016	2	-.003	15	.598	3
64			min	1.579	15	-400.064	3	-55.218	1	-.015	3	-.058	1	-.581	2
65		14	max	33.001	1	132.655	2	3.007	3	.016	2	-.004	15	.841	3
66			min	1.579	15	-223.988	3	-20.762	1	-.015	3	-.088	1	-.735	2
67		15	max	33.001	1	7.853	1	13.694	1	.016	2	-.003	12	.947	3
68			min	1.579	15	-47.913	3	-.051	10	-.015	3	-.091	1	-.788	2
69		16	max	33.001	1	128.162	3	48.151	1	.016	2	0	3	.915	3
70			min	1.579	15	-124.593	2	2.311	15	-.015	3	-.067	1	-.741	2
71		17	max	33.001	1	304.238	3	82.607	1	.016	2	.007	3	.747	3
72			min	1.579	15	-253.217	2	3.908	15	-.015	3	-.016	1	-.594	2
73		18	max	33.001	1	480.313	3	117.064	1	.016	2	.062	1	.442	3
74			min	1.579	15	-381.841	2	5.505	15	-.015	3	.003	15	-.347	2
75		19	max	33.001	1	656.389	3	151.52	1	.016	2	.166	1	0	1
76			min	1.579	15	-510.465	2	7.103	15	-.015	3	.008	15	0	3
77	M15	1	max	-1.665	15	723.616	2	-7.099	15	.017	2	.166	1	0	2
78			min	-34.507	1	-383.645	3	-151.539	1	-.013	3	.008	15	0	3
79		2	max	-1.665	15	531.889	2	-5.501	15	.017	2	.062	1	.261	3
80			min	-34.507	1	-288.702	3	-117.082	1	-.013	3	.003	15	-.488	2
81		3	max	-1.665	15	340.163	2	-3.904	15	.017	2	.007	3	.449	3
82			min	-34.507	1	-193.759	3	-82.626	1	-.013	3	-.016	1	-.827	2
83		4	max	-1.665	15	148.436	2	-2.307	15	.017	2	0	3	.563	3



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-34.507	1	-98.815	3	-48.169	1	-.013	3	-.067	1	-1.017	2
85		5	max	-1.665	15	-.368	15	-.063	10	.017	2	-.003	12	.603	3
86			min	-34.507	1	-43.291	2	-13.713	1	-.013	3	-.091	1	-1.058	2
87		6	max	-1.665	15	91.071	3	20.743	1	.017	2	-.004	15	.569	3
88			min	-34.507	1	-235.018	2	-2.621	3	-.013	3	-.088	1	-.95	2
89		7	max	-1.665	15	186.014	3	55.2	1	.017	2	-.003	15	.461	3
90			min	-34.507	1	-426.744	2	-.186	3	-.013	3	-.058	1	-.693	2
91		8	max	-1.665	15	280.957	3	89.656	1	.017	2	.004	2	.28	3
92			min	-34.507	1	-618.471	2	1.76	12	-.013	3	-.008	3	-.286	2
93		9	max	-1.665	15	375.9	3	124.113	1	.017	2	.081	1	.269	2
94			min	-34.507	1	-810.198	2	3.383	12	-.013	3	-.006	3	.002	15
95		10	max	-1.665	15	1001.925	2	-5.007	12	.013	3	.191	1	.974	2
96			min	-34.507	1	-470.843	3	-158.569	1	-.017	2	-.001	3	-.305	3
97		11	max	-1.665	15	810.198	2	-3.383	12	.013	3	.081	1	.269	2
98			min	-34.507	1	-375.9	3	-124.113	1	-.017	2	-.006	3	.002	15
99		12	max	-1.665	15	618.471	2	-1.76	12	.013	3	.004	2	.28	3
100			min	-34.507	1	-280.957	3	-89.656	1	-.017	2	-.008	3	-.286	2
101		13	max	-1.665	15	426.744	2	.186	3	.013	3	-.003	15	.461	3
102			min	-34.507	1	-186.014	3	-55.2	1	-.017	2	-.058	1	-.693	2
103		14	max	-1.665	15	235.018	2	2.621	3	.013	3	-.004	15	.569	3
104			min	-34.507	1	-91.071	3	-20.743	1	-.017	2	-.088	1	-.95	2
105		15	max	-1.665	15	43.291	2	13.713	1	.013	3	-.003	12	.603	3
106			min	-34.507	1	.368	15	.063	10	-.017	2	-.091	1	-1.058	2
107		16	max	-1.665	15	98.815	3	48.169	1	.013	3	0	3	.563	3
108			min	-34.507	1	-148.436	2	2.307	15	-.017	2	-.067	1	-1.017	2
109		17	max	-1.665	15	193.759	3	82.626	1	.013	3	.007	3	.449	3
110			min	-34.507	1	-340.163	2	3.904	15	-.017	2	-.016	1	-.827	2
111		18	max	-1.665	15	288.702	3	117.082	1	.013	3	.062	1	.261	3
112			min	-34.507	1	-531.889	2	5.501	15	-.017	2	.003	15	-.488	2
113		19	max	-1.665	15	383.645	3	151.539	1	.013	3	.166	1	0	2
114			min	-34.507	1	-723.616	2	7.099	15	-.017	2	.008	15	0	3
115	M16	1	max	-2.638	15	640.122	2	-6.8	15	.006	1	.138	1	0	2
116			min	-55.046	1	-306.932	3	-145.418	1	-.013	3	.007	15	0	3
117		2	max	-2.638	15	448.396	2	-5.203	15	.006	1	.038	1	.202	3
118			min	-55.046	1	-211.989	3	-110.962	1	-.013	3	.002	10	-.423	2
119		3	max	-2.638	15	256.669	2	-3.605	15	.006	1	.003	3	.33	3
120			min	-55.046	1	-117.046	3	-76.505	1	-.013	3	-.035	1	-.697	2
121		4	max	-2.638	15	64.942	2	-2.008	15	.006	1	-.002	12	.384	3
122			min	-55.046	1	-22.103	3	-42.049	1	-.013	3	-.081	1	-.823	2
123		5	max	-2.638	15	72.84	3	.368	10	.006	1	-.004	12	.364	3
124			min	-55.046	1	-126.785	2	-7.592	1	-.013	3	-.1	1	-.799	2
125		6	max	-2.638	15	167.783	3	26.864	1	.006	1	-.004	15	.271	3
126			min	-55.046	1	-318.511	2	-1.187	3	-.013	3	-.093	1	-.625	2
127		7	max	-2.638	15	262.726	3	61.32	1	.006	1	-.003	15	.103	3
128			min	-55.046	1	-510.238	2	1.028	12	-.013	3	-.059	1	-.303	2
129		8	max	-2.638	15	357.669	3	95.777	1	.006	1	.005	2	.168	2
130			min	-55.046	1	-701.965	2	2.652	12	-.013	3	-.007	3	-.138	3
131		9	max	-2.638	15	452.612	3	130.233	1	.006	1	.09	1	.789	2
132			min	-55.046	1	-893.692	2	4.275	12	-.013	3	-.003	3	-.453	3
133		10	max	-2.638	15	1085.419	2	-5.899	12	.013	3	.205	1	1.559	2
134			min	-55.046	1	-547.556	3	-164.69	1	-.006	1	.003	12	-.842	3
135		11	max	-2.638	15	893.692	2	-4.275	12	.013	3	.09	1	.789	2
136			min	-55.046	1	-452.612	3	-130.233	1	-.006	1	-.003	3	-.453	3
137		12	max	-2.638	15	701.965	2	-2.652	12	.013	3	.005	2	.168	2
138			min	-55.046	1	-357.669	3	-95.777	1	-.006	1	-.007	3	-.138	3
139		13	max	-2.638	15	510.238	2	-1.028	12	.013	3	-.003	15	.103	3
140			min	-55.046	1	-262.726	3	-61.32	1	-.006	1	-.059	1	-.303	2



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-2.638	15	318.511	2	1.187	3	.013	3	-.004	15	.271	3
142			min	-55.046	1	-167.783	3	-26.864	1	-.006	1	-.093	1	-.625	2
143		15	max	-2.638	15	126.785	2	7.592	1	.013	3	-.004	12	.364	3
144			min	-55.046	1	-72.84	3	-.368	10	-.006	1	-.1	1	-.799	2
145		16	max	-2.638	15	22.103	3	42.049	1	.013	3	-.002	12	.384	3
146			min	-55.046	1	-64.942	2	2.008	15	-.006	1	-.081	1	-.823	2
147		17	max	-2.638	15	117.046	3	76.505	1	.013	3	.003	3	.33	3
148			min	-55.046	1	-256.669	2	3.605	15	-.006	1	-.035	1	-.697	2
149		18	max	-2.638	15	211.989	3	110.962	1	.013	3	.038	1	.202	3
150			min	-55.046	1	-448.396	2	5.203	15	-.006	1	.002	10	-.423	2
151		19	max	-2.638	15	306.932	3	145.418	1	.013	3	.138	1	0	2
152			min	-55.046	1	-640.122	2	6.8	15	-.006	1	.007	15	0	3
153	M2	1	max	1090.364	2	2.025	4	.267	1	0	3	0	3	0	1
154			min	-1527.922	3	.476	15	.013	15	0	1	0	2	0	1
155		2	max	1090.893	2	1.954	4	.267	1	0	3	0	1	0	15
156			min	-1527.525	3	.459	15	.013	15	0	1	0	15	0	4
157		3	max	1091.422	2	1.883	4	.267	1	0	3	0	1	0	15
158			min	-1527.128	3	.443	15	.013	15	0	1	0	15	-.001	4
159		4	max	1091.951	2	1.812	4	.267	1	0	3	0	1	0	15
160			min	-1526.731	3	.426	15	.013	15	0	1	0	15	-.002	4
161		5	max	1092.481	2	1.74	4	.267	1	0	3	0	1	0	15
162			min	-1526.334	3	.409	15	.013	15	0	1	0	15	-.003	4
163		6	max	1093.01	2	1.669	4	.267	1	0	3	0	1	0	15
164			min	-1525.937	3	.393	15	.013	15	0	1	0	15	-.003	4
165		7	max	1093.539	2	1.598	4	.267	1	0	3	0	1	0	15
166			min	-1525.54	3	.376	15	.013	15	0	1	0	15	-.004	4
167		8	max	1094.069	2	1.527	4	.267	1	0	3	0	1	-.001	15
168			min	-1525.143	3	.359	15	.013	15	0	1	0	15	-.004	4
169		9	max	1094.598	2	1.456	4	.267	1	0	3	0	1	-.001	15
170			min	-1524.746	3	.342	15	.013	15	0	1	0	15	-.005	4
171		10	max	1095.127	2	1.385	4	.267	1	0	3	0	1	-.001	15
172			min	-1524.349	3	.326	15	.013	15	0	1	0	15	-.006	4
173		11	max	1095.656	2	1.314	4	.267	1	0	3	0	1	-.001	15
174			min	-1523.952	3	.309	15	.013	15	0	1	0	15	-.006	4
175		12	max	1096.186	2	1.243	4	.267	1	0	3	.001	1	-.002	15
176			min	-1523.555	3	.292	15	.013	15	0	1	0	15	-.006	4
177		13	max	1096.715	2	1.172	4	.267	1	0	3	.001	1	-.002	15
178			min	-1523.158	3	.269	12	.013	15	0	1	0	15	-.007	4
179		14	max	1097.244	2	1.101	4	.267	1	0	3	.001	1	-.002	15
180			min	-1522.761	3	.241	12	.013	15	0	1	0	15	-.007	4
181		15	max	1097.774	2	1.031	2	.267	1	0	3	.001	1	-.002	15
182			min	-1522.364	3	.214	12	.013	15	0	1	0	15	-.008	4
183		16	max	1098.303	2	.976	2	.267	1	0	3	.001	1	-.002	15
184			min	-1521.967	3	.186	12	.013	15	0	1	0	15	-.008	4
185		17	max	1098.832	2	.921	2	.267	1	0	3	.002	1	-.002	15
186			min	-1521.57	3	.158	12	.013	15	0	1	0	15	-.008	4
187		18	max	1099.361	2	.865	2	.267	1	0	3	.002	1	-.002	15
188			min	-1521.173	3	.131	12	.013	15	0	1	0	15	-.009	4
189		19	max	1099.891	2	.81	2	.267	1	0	3	.002	1	-.002	15
190			min	-1520.776	3	.103	12	.013	15	0	1	0	15	-.009	4
191	M3	1	max	854.314	2	8.876	4	.228	1	0	5	0	1	.009	4
192			min	-986.004	3	2.087	15	.011	15	0	1	0	15	.002	15
193		2	max	854.143	2	8.007	4	.228	1	0	5	0	1	.005	2
194			min	-986.132	3	1.882	15	.011	15	0	1	0	15	0	12
195		3	max	853.973	2	7.138	4	.228	1	0	5	0	1	.002	2
196			min	-986.26	3	1.678	15	.011	15	0	1	0	15	0	3
197		4	max	853.802	2	6.269	4	.228	1	0	5	0	1	0	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-986.388	3	1.474	15	.011	15	0	1	0	15	-.003	3
199		5	max	853.632	2	5.4	4	.228	1	0	5	0	1	-.001	15
200			min	-986.516	3	1.27	15	.011	15	0	1	0	15	-.004	4
201		6	max	853.462	2	4.532	4	.228	1	0	5	0	1	-.002	15
202			min	-986.643	3	1.065	15	.011	15	0	1	0	15	-.007	4
203		7	max	853.291	2	3.663	4	.228	1	0	5	0	1	-.002	15
204			min	-986.771	3	.861	15	.011	15	0	1	0	15	-.009	4
205		8	max	853.121	2	2.794	4	.228	1	0	5	0	1	-.002	15
206			min	-986.899	3	.657	15	.011	15	0	1	0	15	-.01	4
207		9	max	852.951	2	1.925	4	.228	1	0	5	.001	1	-.003	15
208			min	-987.027	3	.453	15	.011	15	0	1	0	15	-.011	4
209		10	max	852.78	2	1.056	4	.228	1	0	5	.001	1	-.003	15
210			min	-987.154	3	.248	15	.011	15	0	1	0	15	-.012	4
211		11	max	852.61	2	.32	2	.228	1	0	5	.001	1	-.003	15
212			min	-987.282	3	-.129	3	.011	15	0	1	0	15	-.012	4
213		12	max	852.44	2	-.16	15	.228	1	0	5	.001	1	-.003	15
214			min	-987.41	3	-.682	4	.011	15	0	1	0	15	-.012	4
215		13	max	852.269	2	-.364	15	.228	1	0	5	.001	1	-.003	15
216			min	-987.538	3	-1.551	4	.011	15	0	1	0	15	-.012	4
217		14	max	852.099	2	-.569	15	.228	1	0	5	.002	1	-.003	15
218			min	-987.665	3	-2.42	4	.011	15	0	1	0	15	-.011	4
219		15	max	851.929	2	-.773	15	.228	1	0	5	.002	1	-.002	15
220			min	-987.793	3	-3.288	4	.011	15	0	1	0	15	-.009	4
221		16	max	851.758	2	-.977	15	.228	1	0	5	.002	1	-.002	15
222			min	-987.921	3	-4.157	4	.011	15	0	1	0	15	-.008	4
223		17	max	851.588	2	-1.181	15	.228	1	0	5	.002	1	-.001	15
224			min	-988.049	3	-5.026	4	.011	15	0	1	0	15	-.006	4
225		18	max	851.418	2	-1.386	15	.228	1	0	5	.002	1	0	15
226			min	-988.176	3	-5.895	4	.011	15	0	1	0	15	-.003	4
227		19	max	851.247	2	-1.59	15	.228	1	0	5	.002	1	0	1
228			min	-988.304	3	-6.764	4	.011	15	0	1	0	15	0	1
229	M4	1	max	918.381	1	0	1	-.418	15	0	1	.002	1	0	1
230			min	-29.246	3	0	1	-8.784	1	0	1	0	15	0	1
231		2	max	918.552	1	0	1	-.418	15	0	1	0	1	0	1
232			min	-29.118	3	0	1	-8.784	1	0	1	0	15	0	1
233		3	max	918.722	1	0	1	-.418	15	0	1	0	15	0	1
234			min	-28.99	3	0	1	-8.784	1	0	1	0	1	0	1
235		4	max	918.892	1	0	1	-.418	15	0	1	0	15	0	1
236			min	-28.862	3	0	1	-8.784	1	0	1	-.001	1	0	1
237		5	max	919.063	1	0	1	-.418	15	0	1	0	15	0	1
238			min	-28.735	3	0	1	-8.784	1	0	1	-.002	1	0	1
239		6	max	919.233	1	0	1	-.418	15	0	1	0	15	0	1
240			min	-28.607	3	0	1	-8.784	1	0	1	-.003	1	0	1
241		7	max	919.403	1	0	1	-.418	15	0	1	0	15	0	1
242			min	-28.479	3	0	1	-8.784	1	0	1	-.004	1	0	1
243		8	max	919.574	1	0	1	-.418	15	0	1	0	15	0	1
244			min	-28.351	3	0	1	-8.784	1	0	1	-.005	1	0	1
245		9	max	919.744	1	0	1	-.418	15	0	1	0	15	0	1
246			min	-28.224	3	0	1	-8.784	1	0	1	-.006	1	0	1
247		10	max	919.914	1	0	1	-.418	15	0	1	0	15	0	1
248			min	-28.096	3	0	1	-8.784	1	0	1	-.007	1	0	1
249		11	max	920.085	1	0	1	-.418	15	0	1	0	15	0	1
250			min	-27.968	3	0	1	-8.784	1	0	1	-.008	1	0	1
251		12	max	920.255	1	0	1	-.418	15	0	1	0	15	0	1
252			min	-27.84	3	0	1	-8.784	1	0	1	-.009	1	0	1
253		13	max	920.425	1	0	1	-.418	15	0	1	0	15	0	1
254			min	-27.712	3	0	1	-8.784	1	0	1	-.01	1	0	1



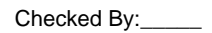
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	920.596	1	0	1	-418	15	0	1	0	15	0	1
256		min	-27.585	3	0	1	-8.784	1	0	1	-.011	1	0	1
257	15	max	920.766	1	0	1	-418	15	0	1	0	15	0	1
258		min	-27.457	3	0	1	-8.784	1	0	1	-.012	1	0	1
259	16	max	920.936	1	0	1	-418	15	0	1	0	15	0	1
260		min	-27.329	3	0	1	-8.784	1	0	1	-.013	1	0	1
261	17	max	921.107	1	0	1	-418	15	0	1	0	15	0	1
262		min	-27.201	3	0	1	-8.784	1	0	1	-.014	1	0	1
263	18	max	921.277	1	0	1	-418	15	0	1	0	15	0	1
264		min	-27.074	3	0	1	-8.784	1	0	1	-.015	1	0	1
265	19	max	921.447	1	0	1	-418	15	0	1	0	15	0	1
266		min	-26.946	3	0	1	-8.784	1	0	1	-.016	1	0	1
267	M6	1	max	3238.862	2	2.295	2	0	1	0	1	0	1	1
268		min	-4677.703	3	.23	12	0	1	0	1	0	1	0	1
269	2	max	3239.391	2	2.239	2	0	1	0	1	0	1	0	12
270		min	-4677.306	3	.203	12	0	1	0	1	0	1	0	2
271	3	max	3239.921	2	2.184	2	0	1	0	1	0	1	0	12
272		min	-4676.909	3	.17	3	0	1	0	1	0	1	-.002	2
273	4	max	3240.45	2	2.129	2	0	1	0	1	0	1	0	12
274		min	-4676.512	3	.128	3	0	1	0	1	0	1	-.002	2
275	5	max	3240.979	2	2.073	2	0	1	0	1	0	1	0	3
276		min	-4676.115	3	.087	3	0	1	0	1	0	1	-.003	2
277	6	max	3241.509	2	2.018	2	0	1	0	1	0	1	0	3
278		min	-4675.718	3	.045	3	0	1	0	1	0	1	-.004	2
279	7	max	3242.038	2	1.963	2	0	1	0	1	0	1	0	3
280		min	-4675.321	3	.004	3	0	1	0	1	0	1	-.005	2
281	8	max	3242.567	2	1.907	2	0	1	0	1	0	1	0	3
282		min	-4674.924	3	-.038	3	0	1	0	1	0	1	-.005	2
283	9	max	3243.096	2	1.852	2	0	1	0	1	0	1	0	3
284		min	-4674.527	3	-.079	3	0	1	0	1	0	1	-.006	2
285	10	max	3243.626	2	1.797	2	0	1	0	1	0	1	0	3
286		min	-4674.13	3	-.121	3	0	1	0	1	0	1	-.007	2
287	11	max	3244.155	2	1.741	2	0	1	0	1	0	1	0	3
288		min	-4673.733	3	-.162	3	0	1	0	1	0	1	-.007	2
289	12	max	3244.684	2	1.686	2	0	1	0	1	0	1	0	3
290		min	-4673.336	3	-.204	3	0	1	0	1	0	1	-.008	2
291	13	max	3245.214	2	1.63	2	0	1	0	1	0	1	0	3
292		min	-4672.939	3	-.245	3	0	1	0	1	0	1	-.008	2
293	14	max	3245.743	2	1.575	2	0	1	0	1	0	1	0	3
294		min	-4672.542	3	-.287	3	0	1	0	1	0	1	-.009	2
295	15	max	3246.272	2	1.52	2	0	1	0	1	0	1	0	3
296		min	-4672.145	3	-.328	3	0	1	0	1	0	1	-.01	2
297	16	max	3246.802	2	1.464	2	0	1	0	1	0	1	0	3
298		min	-4671.748	3	-.37	3	0	1	0	1	0	1	-.01	2
299	17	max	3247.331	2	1.409	2	0	1	0	1	0	1	0	3
300		min	-4671.351	3	-.411	3	0	1	0	1	0	1	-.011	2
301	18	max	3247.86	2	1.354	2	0	1	0	1	0	1	0	3
302		min	-4670.954	3	-.453	3	0	1	0	1	0	1	-.011	2
303	19	max	3248.389	2	1.298	2	0	1	0	1	0	1	0	3
304		min	-4670.557	3	-.494	3	0	1	0	1	0	1	-.012	2
305	M7	1	max	2734.373	2	8.897	4	0	1	0	1	0	.012	2
306		min	-2881.506	3	2.09	15	0	1	0	1	0	1	0	3
307	2	max	2734.203	2	8.028	4	0	1	0	1	0	1	.008	2
308		min	-2881.634	3	1.885	15	0	1	0	1	0	1	-.003	3
309	3	max	2734.032	2	7.159	4	0	1	0	1	0	1	.005	2
310		min	-2881.762	3	1.681	15	0	1	0	1	0	1	-.005	3
311	4	max	2733.862	2	6.29	4	0	1	0	1	0	1	.002	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2440.615	2	0	1	0	1	0	1	0	1	0	1
370			min	-186.873	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2440.785	2	0	1	0	1	0	1	0	1	0	1
372			min	-186.746	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2440.955	2	0	1	0	1	0	1	0	1	0	1
374			min	-186.618	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2441.126	2	0	1	0	1	0	1	0	1	0	1
376			min	-186.49	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2441.296	2	0	1	0	1	0	1	0	1	0	1
378			min	-186.362	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2441.466	2	0	1	0	1	0	1	0	1	0	1
380			min	-186.235	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1090.364	2	2.025	4	-.013	15	0	1	0	2	0	1
382			min	-1527.922	3	.476	15	-.267	1	0	3	0	3	0	1
383		2	max	1090.893	2	1.954	4	-.013	15	0	1	0	15	0	15
384			min	-1527.525	3	.459	15	-.267	1	0	3	0	1	0	4
385		3	max	1091.422	2	1.883	4	-.013	15	0	1	0	15	0	15
386			min	-1527.128	3	.443	15	-.267	1	0	3	0	1	-.001	4
387		4	max	1091.951	2	1.812	4	-.013	15	0	1	0	15	0	15
388			min	-1526.731	3	.426	15	-.267	1	0	3	0	1	-.002	4
389		5	max	1092.481	2	1.74	4	-.013	15	0	1	0	15	0	15
390			min	-1526.334	3	.409	15	-.267	1	0	3	0	1	-.003	4
391		6	max	1093.01	2	1.669	4	-.013	15	0	1	0	15	0	15
392			min	-1525.937	3	.393	15	-.267	1	0	3	0	1	-.003	4
393		7	max	1093.539	2	1.598	4	-.013	15	0	1	0	15	0	15
394			min	-1525.54	3	.376	15	-.267	1	0	3	0	1	-.004	4
395		8	max	1094.069	2	1.527	4	-.013	15	0	1	0	15	-.001	15
396			min	-1525.143	3	.359	15	-.267	1	0	3	0	1	-.004	4
397		9	max	1094.598	2	1.456	4	-.013	15	0	1	0	15	-.001	15
398			min	-1524.746	3	.342	15	-.267	1	0	3	0	1	-.005	4
399		10	max	1095.127	2	1.385	4	-.013	15	0	1	0	15	-.001	15
400			min	-1524.349	3	.326	15	-.267	1	0	3	0	1	-.006	4
401		11	max	1095.656	2	1.314	4	-.013	15	0	1	0	15	-.001	15
402			min	-1523.952	3	.309	15	-.267	1	0	3	0	1	-.006	4
403		12	max	1096.186	2	1.243	4	-.013	15	0	1	0	15	-.002	15
404			min	-1523.555	3	.292	15	-.267	1	0	3	-.001	1	-.006	4
405		13	max	1096.715	2	1.172	4	-.013	15	0	1	0	15	-.002	15
406			min	-1523.158	3	.269	12	-.267	1	0	3	-.001	1	-.007	4
407		14	max	1097.244	2	1.101	4	-.013	15	0	1	0	15	-.002	15
408			min	-1522.761	3	.241	12	-.267	1	0	3	-.001	1	-.007	4
409		15	max	1097.774	2	1.031	2	-.013	15	0	1	0	15	-.002	15
410			min	-1522.364	3	.214	12	-.267	1	0	3	-.001	1	-.008	4
411		16	max	1098.303	2	.976	2	-.013	15	0	1	0	15	-.002	15
412			min	-1521.967	3	.186	12	-.267	1	0	3	-.001	1	-.008	4
413		17	max	1098.832	2	.921	2	-.013	15	0	1	0	15	-.002	15
414			min	-1521.57	3	.158	12	-.267	1	0	3	-.002	1	-.008	4
415		18	max	1099.361	2	.865	2	-.013	15	0	1	0	15	-.002	15
416			min	-1521.173	3	.131	12	-.267	1	0	3	-.002	1	-.009	4
417		19	max	1099.891	2	.81	2	-.013	15	0	1	0	15	-.002	15
418			min	-1520.776	3	.103	12	-.267	1	0	3	-.002	1	-.009	4
419	M11	1	max	854.314	2	8.876	4	-.011	15	0	1	0	15	.009	4
420			min	-986.004	3	2.087	15	-.228	1	0	5	0	1	.002	15
421		2	max	854.143	2	8.007	4	-.011	15	0	1	0	15	.005	2
422			min	-986.132	3	1.882	15	-.228	1	0	5	0	1	0	12
423		3	max	853.973	2	7.138	4	-.011	15	0	1	0	15	.002	2
424			min	-986.26	3	1.678	15	-.228	1	0	5	0	1	0	3
425		4	max	853.802	2	6.269	4	-.011	15	0	1	0	15	0	2





Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1172.471	2	-1749.563	3	0	1	0	1	0	1	-2.047	3
541		5	max	1919.302	3	1442.734	2	0	1	0	1	0	1	.062	1
542			min	-1171.629	2	-1750.657	3	0	1	0	1	0	1	-.961	3
543		6	max	1919.933	3	1441.275	2	0	1	0	1	0	1	.126	3
544			min	-1170.786	2	-1751.752	3	0	1	0	1	0	1	-.87	2
545		7	max	1920.565	3	1439.816	2	0	1	0	1	0	1	1.214	3
546			min	-1169.944	2	-1752.846	3	0	1	0	1	0	1	-1.764	2
547		8	max	1921.197	3	1438.357	2	0	1	0	1	0	1	2.302	3
548			min	-1169.101	2	-1753.94	3	0	1	0	1	0	1	-2.657	2
549		9	max	1935.3	3	180.594	2	0	1	0	1	0	1	2.653	3
550			min	-1030.293	2	.438	15	0	1	0	1	0	1	-3.05	2
551		10	max	1935.932	3	179.135	2	0	1	0	1	0	1	2.562	3
552			min	-1029.451	2	-.002	15	0	1	0	1	0	1	-3.161	2
553		11	max	1936.564	3	177.676	2	0	1	0	1	0	1	2.472	3
554			min	-1028.608	2	-1.703	4	0	1	0	1	0	1	-3.272	2
555		12	max	1951.433	3	1137.425	3	0	1	0	1	0	1	2.161	3
556			min	-890.136	2	-1798.155	2	0	1	0	1	0	1	-2.925	2
557		13	max	1952.065	3	1136.331	3	0	1	0	1	0	1	1.456	3
558			min	-889.293	2	-1799.614	2	0	1	0	1	0	1	-1.808	2
559		14	max	1952.697	3	1135.237	3	0	1	0	1	0	1	.751	3
560			min	-888.451	2	-1801.073	2	0	1	0	1	0	1	-.691	2
561		15	max	1953.328	3	1134.142	3	0	1	0	1	0	1	.427	2
562			min	-887.609	2	-1802.532	2	0	1	0	1	0	1	0	15
563		16	max	1953.96	3	1133.048	3	0	1	0	1	0	1	1.547	2
564			min	-886.766	2	-1803.991	2	0	1	0	1	0	1	-.657	3
565		17	max	1954.592	3	1131.954	3	0	1	0	1	0	1	2.667	2
566			min	-885.924	2	-1805.45	2	0	1	0	1	0	1	-1.36	3
567		18	max	-12.218	12	2175.062	2	0	1	0	1	0	1	1.36	2
568			min	-330.231	1	-1094.416	3	0	1	0	1	0	1	-.705	3
569		19	max	-11.797	12	2173.603	2	0	1	0	1	0	1	.012	1
570			min	-329.388	1	-1095.511	3	0	1	0	1	0	1	-.026	3
571	M9	1	max	144.751	1	784.309	3	48.36	1	0	3	-.007	15	0	15
572			min	6.784	15	-420.817	2	2.33	15	0	2	-.135	1	-.013	2
573		2	max	145.593	1	783.215	3	48.36	1	0	3	-.005	15	.249	2
574			min	7.038	15	-422.276	2	2.33	15	0	2	-.105	1	-.493	3
575		3	max	638.488	3	565.734	2	48.197	1	0	2	-.004	15	.5	2
576			min	-382.81	2	-620.383	3	2.317	15	0	3	-.075	1	-.964	3
577		4	max	639.12	3	564.275	2	48.197	1	0	2	-.002	15	.15	2
578			min	-381.968	2	-621.478	3	2.317	15	0	3	-.045	1	-.578	3
579		5	max	639.751	3	562.816	2	48.197	1	0	2	0	15	-.005	15
580			min	-381.125	2	-622.572	3	2.317	15	0	3	-.015	1	-.2	2
581		6	max	640.383	3	561.357	2	48.197	1	0	2	.015	1	.194	3
582			min	-380.283	2	-623.666	3	2.317	15	0	3	0	15	-.549	2
583		7	max	641.015	3	559.898	2	48.197	1	0	2	.045	1	.582	3
584			min	-379.441	2	-624.76	3	2.317	15	0	3	.002	15	-.897	2
585		8	max	641.647	3	558.439	2	48.197	1	0	2	.075	1	.97	3
586			min	-378.598	2	-625.855	3	2.317	15	0	3	.004	15	-1.244	2
587		9	max	657.461	3	52.759	2	81.236	1	0	3	-.002	15	1.127	3
588			min	-317.065	2	.446	15	3.896	15	0	9	-.05	1	-1.418	2
589		10	max	658.093	3	51.3	2	81.236	1	0	3	0	1	1.105	3
590			min	-316.223	2	.005	15	3.896	15	0	9	0	10	-1.451	2
591		11	max	658.725	3	49.841	2	81.236	1	0	3	.051	1	1.083	3
592			min	-315.38	2	-1.783	4	3.896	15	0	9	.002	15	-1.482	2
593		12	max	674.156	3	425.206	3	46.74	1	0	3	-.004	15	.952	3
594			min	-253.68	2	-671.281	2	2.23	15	0	2	-.073	1	-1.317	2
595		13	max	674.788	3	424.111	3	46.74	1	0	3	-.002	15	.688	3
596			min	-252.837	2	-672.74	2	2.23	15	0	2	-.044	1	-.9	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	675.419	3	423.017	3	46.74	1	0	3	0	15	.425	3
598		min	-251.995	2	-674.199	2	2.23	15	0	2	-.015	1	-.482	2
599	15	max	676.051	3	421.923	3	46.74	1	0	3	.014	1	.163	3
600		min	-251.153	2	-675.658	2	2.23	15	0	2	0	15	-.078	1
601	16	max	676.683	3	420.828	3	46.74	1	0	3	.043	1	.357	2
602		min	-250.31	2	-677.117	2	2.23	15	0	2	.002	15	-.098	3
603	17	max	677.315	3	419.734	3	46.74	1	0	3	.072	1	.778	2
604		min	-249.468	2	-678.576	2	2.23	15	0	2	.003	15	-.359	3
605	18	max	-7.054	15	642.39	2	55.108	1	0	2	.103	1	.393	2
606		min	-146.256	1	-305.995	3	2.638	15	0	3	.005	15	-.177	3
607	19	max	-6.8	15	640.931	2	55.108	1	0	2	.138	1	.013	3
608		min	-145.413	1	-307.089	3	2.638	15	0	3	.007	15	-.006	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.224	2	.012	3	1.546e-2	2	NC	1	NC	1
2			min	0	15	-.072	3	-.008	2	-4.977e-3	3	NC	1	NC	1
3		2	max	0	1	.172	2	.015	3	1.634e-2	2	NC	4	NC	1
4			min	0	15	.004	15	-.004	10	-4.499e-3	3	1226.283	3	NC	1
5		3	max	0	1	.178	3	.029	1	1.722e-2	2	NC	4	NC	2
6			min	0	15	.003	15	-.003	10	-4.022e-3	3	672.741	3	5555.388	1
7		4	max	0	1	.25	3	.042	1	1.811e-2	2	NC	5	NC	2
8			min	0	15	.003	15	-.002	10	-3.544e-3	3	522.098	3	3827.726	1
9		5	max	0	1	.274	3	.048	1	1.899e-2	2	NC	5	NC	2
10			min	0	15	.003	15	-.003	10	-3.066e-3	3	486.452	3	3371.118	1
11		6	max	0	1	.249	3	.045	1	1.987e-2	2	NC	4	NC	2
12			min	0	15	.003	15	-.005	10	-2.589e-3	3	522.954	3	3631.358	1
13		7	max	0	1	.208	2	.033	3	2.075e-2	2	NC	2	NC	2
14			min	0	15	.004	15	-.008	10	-2.111e-3	3	649.035	3	4948.538	1
15		8	max	0	1	.266	2	.035	3	2.163e-2	2	NC	4	NC	1
16			min	0	15	.005	15	-.013	2	-1.634e-3	3	953.918	3	7508.06	3
17		9	max	0	1	.316	2	.035	3	2.252e-2	2	NC	4	NC	1
18			min	0	15	.006	15	-.021	2	-1.156e-3	3	1686.184	3	7259.634	3
19		10	max	0	1	.339	2	.036	3	2.34e-2	2	NC	4	NC	1
20		min	0	1	-.007	3	-.025	2	-6.783e-4	3	1460.309	2	7207.347	3	
21	11	max	0	15	.316	2	.035	3	2.252e-2	2	NC	4	NC	1	
22		min	0	1	.006	15	-.021	2	-1.156e-3	3	1686.184	3	7259.634	3	
23	12	max	0	15	.266	2	.035	3	2.163e-2	2	NC	4	NC	1	
24		min	0	1	.005	15	-.013	2	-1.634e-3	3	953.918	3	7508.06	3	
25	13	max	0	15	.208	2	.033	3	2.075e-2	2	NC	2	NC	2	
26		min	0	1	.004	15	-.008	10	-2.111e-3	3	649.035	3	4948.538	1	
27	14	max	0	15	.249	3	.045	1	1.987e-2	2	NC	4	NC	2	
28		min	0	1	.003	15	-.005	10	-2.589e-3	3	522.954	3	3631.358	1	
29	15	max	0	15	.274	3	.048	1	1.899e-2	2	NC	5	NC	2	
30		min	0	1	.003	15	-.003	10	-3.066e-3	3	486.452	3	3371.118	1	
31	16	max	0	15	.25	3	.042	1	1.811e-2	2	NC	5	NC	2	
32		min	0	1	.003	15	-.002	10	-3.544e-3	3	522.098	3	3827.726	1	
33	17	max	0	15	.178	3	.029	1	1.722e-2	2	NC	4	NC	2	
34		min	0	1	.003	15	-.003	10	-4.022e-3	3	672.741	3	5555.388	1	
35	18	max	0	15	.172	2	.015	3	1.634e-2	2	NC	4	NC	1	
36		min	0	1	.004	15	-.004	10	-4.499e-3	3	1226.283	3	NC	1	
37	19	max	0	15	.224	2	.012	3	1.546e-2	2	NC	1	NC	1	
38		min	0	1	-.072	3	-.008	2	-4.977e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.478	3	.011	3	8.425e-3	2	NC	1	NC	1
40			min	0	15	-.662	2	-.007	2	-7.049e-3	3	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.668	3	.012	3	9.524e-3	2	NC	5	NC	1
42			min	0	15	-.847	2	-.004	2	-8.089e-3	3	885.513	3	NC	1
43		3	max	0	1	.837	3	.021	1	1.062e-2	2	NC	5	NC	2
44			min	0	15	-1.016	2	-.003	10	-9.129e-3	3	468.269	3	7523.835	1
45		4	max	0	1	.971	3	.034	1	1.172e-2	2	NC	5	NC	2
46			min	0	15	-1.157	2	-.002	10	-1.017e-2	3	339.226	2	4760.286	1
47		5	max	0	1	1.061	3	.041	1	1.282e-2	2	NC	15	NC	2
48			min	0	15	-1.263	2	-.003	10	-1.121e-2	3	279.535	2	3995.013	1
49		6	max	0	1	1.107	3	.039	1	1.392e-2	2	NC	15	NC	2
50			min	0	15	-1.331	2	-.004	10	-1.225e-2	3	250.995	2	4169.722	1
51		7	max	0	1	1.112	3	.029	1	1.502e-2	2	NC	15	NC	2
52			min	0	15	-1.365	2	-.007	10	-1.329e-2	3	239.042	2	5549.832	1
53		8	max	0	1	1.09	3	.03	3	1.612e-2	2	NC	15	NC	1
54			min	0	15	-1.371	2	-.012	2	-1.433e-2	3	237.032	2	8581.25	3
55		9	max	0	1	1.057	3	.031	3	1.722e-2	2	NC	15	NC	1
56			min	0	15	-1.361	2	-.019	2	-1.537e-2	3	240.216	2	8236.34	3
57		10	max	0	1	1.04	3	.031	3	1.832e-2	2	NC	15	NC	1
58			min	0	1	-1.353	2	-.023	2	-1.641e-2	3	242.922	2	8157.554	3
59		11	max	0	15	1.057	3	.031	3	1.722e-2	2	NC	15	NC	1
60			min	0	1	-1.361	2	-.019	2	-1.537e-2	3	240.216	2	8236.34	3
61		12	max	0	15	1.09	3	.03	3	1.612e-2	2	NC	15	NC	1
62			min	0	1	-1.371	2	-.012	2	-1.433e-2	3	237.032	2	8581.25	3
63		13	max	0	15	1.112	3	.029	1	1.502e-2	2	NC	15	NC	2
64			min	0	1	-1.365	2	-.007	10	-1.329e-2	3	239.042	2	5549.832	1
65		14	max	0	15	1.107	3	.039	1	1.392e-2	2	NC	15	NC	2
66			min	0	1	-1.331	2	-.004	10	-1.225e-2	3	250.995	2	4169.722	1
67		15	max	0	15	1.061	3	.041	1	1.282e-2	2	NC	15	NC	2
68			min	0	1	-1.263	2	-.003	10	-1.121e-2	3	279.535	2	3995.013	1
69		16	max	0	15	.971	3	.034	1	1.172e-2	2	NC	5	NC	2
70			min	0	1	-1.157	2	-.002	10	-1.017e-2	3	339.226	2	4760.286	1
71		17	max	0	15	.837	3	.021	1	1.062e-2	2	NC	5	NC	2
72			min	0	1	-1.016	2	-.003	10	-9.129e-3	3	468.269	3	7523.835	1
73		18	max	0	15	.668	3	.012	3	9.524e-3	2	NC	5	NC	1
74			min	0	1	-.847	2	-.004	2	-8.089e-3	3	885.513	3	NC	1
75		19	max	0	15	.478	3	.011	3	8.425e-3	2	NC	1	NC	1
76			min	0	1	-.662	2	-.007	2	-7.049e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.488	3	.01	3	6.064e-3	3	NC	1	NC	1
78			min	0	1	-.66	2	-.007	2	-8.785e-3	2	NC	1	NC	1
79		2	max	0	15	.635	3	.011	3	6.946e-3	3	NC	5	NC	1
80			min	0	1	-.88	2	-.004	2	-9.941e-3	2	763.717	2	NC	1
81		3	max	0	15	.77	3	.022	1	7.827e-3	3	NC	5	NC	2
82			min	0	1	-1.077	2	-.002	10	-1.11e-2	2	402.77	2	7466.674	1
83		4	max	0	15	.884	3	.035	1	8.709e-3	3	NC	5	NC	2
84			min	0	1	-1.235	2	-.002	10	-1.225e-2	2	292.11	2	4727.161	1
85		5	max	0	15	.971	3	.041	1	9.591e-3	3	NC	15	NC	2
86			min	0	1	-1.345	2	-.002	10	-1.341e-2	2	245.293	2	3965.241	1
87		6	max	0	15	1.029	3	.04	1	1.047e-2	3	NC	15	NC	2
88			min	0	1	-1.405	2	-.004	10	-1.456e-2	2	225.667	2	4131.285	1
89		7	max	0	15	1.06	3	.03	1	1.135e-2	3	NC	15	NC	2
90			min	0	1	-1.419	2	-.006	10	-1.572e-2	2	221.34	2	5473.301	1
91		8	max	0	15	1.069	3	.028	3	1.224e-2	3	NC	15	NC	1
92			min	0	1	-1.401	2	-.011	2	-1.688e-2	2	226.747	2	9234.658	3
93		9	max	0	15	1.066	3	.029	3	1.312e-2	3	NC	15	NC	1
94			min	0	1	-1.37	2	-.018	2	-1.803e-2	2	236.731	2	8909.626	3
95		10	max	0	1	1.061	3	.029	3	1.4e-2	3	NC	15	NC	1
96			min	0	1	-1.352	2	-.022	2	-1.919e-2	2	242.792	2	8844.889	3
97		11	max	0	1	1.066	3	.029	3	1.312e-2	3	NC	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.37	2	-.018	2	-1.803e-2	2	236.731	2	8909.626	3
99		12	max	0	1	1.069	3	.028	3	1.224e-2	3	NC	15	NC	1
100			min	0	15	-1.401	2	-.011	2	-1.688e-2	2	226.747	2	9234.658	3
101		13	max	0	1	1.06	3	.03	1	1.135e-2	3	NC	15	NC	2
102			min	0	15	-1.419	2	-.006	10	-1.572e-2	2	221.34	2	5473.301	1
103		14	max	0	1	1.029	3	.04	1	1.047e-2	3	NC	15	NC	2
104			min	0	15	-1.405	2	-.004	10	-1.456e-2	2	225.667	2	4131.285	1
105		15	max	0	1	.971	3	.041	1	9.591e-3	3	NC	15	NC	2
106			min	0	15	-1.345	2	-.002	10	-1.341e-2	2	245.293	2	3965.241	1
107		16	max	0	1	.884	3	.035	1	8.709e-3	3	NC	5	NC	2
108			min	0	15	-1.235	2	-.002	10	-1.225e-2	2	292.11	2	4727.161	1
109		17	max	0	1	.77	3	.022	1	7.827e-3	3	NC	5	NC	2
110			min	0	15	-1.077	2	-.002	10	-1.11e-2	2	402.77	2	7466.674	1
111		18	max	0	1	.635	3	.011	3	6.946e-3	3	NC	5	NC	1
112			min	0	15	-.88	2	-.004	2	-9.941e-3	2	763.717	2	NC	1
113		19	max	0	1	.488	3	.01	3	6.064e-3	3	NC	1	NC	1
114			min	0	15	-.66	2	-.007	2	-8.785e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.199	2	.009	3	1.171e-2	3	NC	1	NC	1
116			min	0	1	-.173	3	-.006	2	-1.299e-2	2	NC	1	NC	1
117		2	max	0	15	.105	2	.012	1	1.259e-2	3	NC	4	NC	1
118			min	0	1	-.142	3	-.003	10	-1.334e-2	2	1785.775	2	NC	1
119		3	max	0	15	.049	1	.029	1	1.347e-2	3	NC	4	NC	2
120			min	0	1	-.12	3	-.001	10	-1.37e-2	2	997.451	2	5551.06	1
121		4	max	0	15	.023	9	.043	1	1.434e-2	3	NC	4	NC	2
122			min	0	1	-.113	3	0	10	-1.406e-2	2	800.399	2	3803.114	1
123		5	max	0	15	.025	9	.049	1	1.522e-2	3	NC	4	NC	2
124			min	0	1	-.125	3	0	10	-1.442e-2	2	790.699	2	3328.175	1
125		6	max	0	15	.051	1	.046	1	1.61e-2	3	NC	4	NC	2
126			min	0	1	-.154	3	-.002	10	-1.477e-2	2	947.498	2	3550.762	1
127		7	max	0	15	.101	1	.034	1	1.697e-2	3	NC	4	NC	2
128			min	0	1	-.195	3	-.005	10	-1.513e-2	2	1490.358	2	4745.197	1
129		8	max	0	15	.164	2	.025	3	1.785e-2	3	NC	1	NC	2
130			min	0	1	-.242	3	-.008	10	-1.549e-2	2	2463.697	3	9303.698	1
131		9	max	0	15	.234	2	.025	3	1.873e-2	3	NC	4	NC	1
132			min	0	1	-.281	3	-.016	2	-1.585e-2	2	1563.745	3	NC	1
133		10	max	0	1	.266	2	.025	3	1.96e-2	3	NC	4	NC	1
134			min	0	1	-.298	3	-.02	2	-1.621e-2	2	1348.034	3	NC	1
135		11	max	0	1	.234	2	.025	3	1.873e-2	3	NC	4	NC	1
136			min	0	15	-.281	3	-.016	2	-1.585e-2	2	1563.745	3	NC	1
137		12	max	0	1	.164	2	.025	3	1.785e-2	3	NC	1	NC	2
138			min	0	15	-.242	3	-.008	10	-1.549e-2	2	2463.697	3	9303.698	1
139		13	max	0	1	.101	1	.034	1	1.697e-2	3	NC	4	NC	2
140			min	0	15	-.195	3	-.005	10	-1.513e-2	2	1490.358	2	4745.197	1
141		14	max	0	1	.051	1	.046	1	1.61e-2	3	NC	4	NC	2
142			min	0	15	-.154	3	-.002	10	-1.477e-2	2	947.498	2	3550.762	1
143		15	max	0	1	.025	9	.049	1	1.522e-2	3	NC	4	NC	2
144			min	0	15	-.125	3	0	10	-1.442e-2	2	790.699	2	3328.175	1
145		16	max	0	1	.023	9	.043	1	1.434e-2	3	NC	4	NC	2
146			min	0	15	-.113	3	0	10	-1.406e-2	2	800.399	2	3803.114	1
147		17	max	0	1	.049	1	.029	1	1.347e-2	3	NC	4	NC	2
148			min	0	15	-.12	3	-.001	10	-1.37e-2	2	997.451	2	5551.06	1
149		18	max	0	1	.105	2	.012	1	1.259e-2	3	NC	4	NC	1
150			min	0	15	-.142	3	-.003	10	-1.334e-2	2	1785.775	2	NC	1
151		19	max	0	1	.199	2	.009	3	1.171e-2	3	NC	1	NC	1
152			min	0	15	-.173	3	-.006	2	-1.299e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.012	2	.006	1	-6.696e-6	15	NC	1	NC	1
154			min	-.011	3	-.018	3	0	15	-1.387e-4	1	6404.038	2	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.008	2	.01	2	.006	1	-6.369e-6	15	NC	1	NC	1
156			min	-.011	3	-.018	3	0	15	-1.32e-4	1	7422.371	2	NC	1
157		3	max	.007	2	.009	2	.005	1	-6.041e-6	15	NC	1	NC	1
158			min	-.01	3	-.017	3	0	15	-1.252e-4	1	8804.824	2	NC	1
159		4	max	.007	2	.007	2	.005	1	-5.714e-6	15	NC	1	NC	1
160			min	-.009	3	-.017	3	0	15	-1.184e-4	1	NC	1	NC	1
161		5	max	.006	2	.006	2	.004	1	-5.386e-6	15	NC	1	NC	1
162			min	-.009	3	-.016	3	0	15	-1.116e-4	1	NC	1	NC	1
163		6	max	.006	2	.004	2	.004	1	-5.059e-6	15	NC	1	NC	1
164			min	-.008	3	-.015	3	0	15	-1.048e-4	1	NC	1	NC	1
165		7	max	.005	2	.003	2	.003	1	-4.732e-6	15	NC	1	NC	1
166			min	-.008	3	-.015	3	0	15	-9.799e-5	1	NC	1	NC	1
167		8	max	.005	2	.002	2	.003	1	-4.404e-6	15	NC	1	NC	1
168			min	-.007	3	-.014	3	0	15	-9.12e-5	1	NC	1	NC	1
169		9	max	.005	2	0	2	.002	1	-4.077e-6	15	NC	1	NC	1
170			min	-.006	3	-.013	3	0	15	-8.441e-5	1	NC	1	NC	1
171		10	max	.004	2	0	2	.002	1	-3.749e-6	15	NC	1	NC	1
172			min	-.006	3	-.012	3	0	15	-7.762e-5	1	NC	1	NC	1
173		11	max	.004	2	-.001	2	.002	1	-3.422e-6	15	NC	1	NC	1
174			min	-.005	3	-.011	3	0	15	-7.083e-5	1	NC	1	NC	1
175		12	max	.003	2	-.002	15	.001	1	-3.095e-6	15	NC	1	NC	1
176			min	-.004	3	-.01	3	0	15	-6.404e-5	1	NC	1	NC	1
177		13	max	.003	2	-.002	15	0	1	-2.767e-6	15	NC	1	NC	1
178			min	-.004	3	-.009	3	0	15	-5.724e-5	1	NC	1	NC	1
179		14	max	.002	2	-.002	15	0	1	-2.44e-6	15	NC	1	NC	1
180			min	-.003	3	-.008	3	0	15	-5.045e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-2.113e-6	15	NC	1	NC	1
182			min	-.003	3	-.006	3	0	15	-4.366e-5	1	NC	1	NC	1
183		16	max	.001	2	-.001	15	0	1	-1.785e-6	15	NC	1	NC	1
184			min	-.002	3	-.005	3	0	15	-3.687e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-1.458e-6	15	NC	1	NC	1
186			min	-.001	3	-.003	4	0	15	-3.008e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-1.13e-6	15	NC	1	NC	1
188			min	0	3	-.002	4	0	15	-2.329e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-8.029e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.649e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	3.342e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	1.631e-7	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	1.952e-5	1	NC	1	NC	1
194			min	0	2	-.003	4	0	1	9.322e-7	15	NC	1	NC	1
195		3	max	.001	3	-.001	15	0	15	3.57e-5	1	NC	1	NC	1
196			min	0	2	-.006	4	0	1	1.701e-6	15	NC	1	NC	1
197		4	max	.002	3	-.002	15	0	15	5.188e-5	1	NC	1	NC	1
198			min	-.001	2	-.009	4	0	1	2.47e-6	15	NC	1	NC	1
199		5	max	.002	3	-.003	15	0	15	6.806e-5	1	NC	1	NC	1
200			min	-.002	2	-.012	4	0	1	3.239e-6	15	8392.885	4	NC	1
201		6	max	.003	3	-.004	15	0	15	8.424e-5	1	NC	2	NC	1
202			min	-.002	2	-.015	4	0	1	4.009e-6	15	6810.205	4	NC	1
203		7	max	.003	3	-.004	15	0	15	1.004e-4	1	NC	5	NC	1
204			min	-.003	2	-.018	4	0	1	4.778e-6	15	5856.599	4	NC	1
205		8	max	.004	3	-.005	15	0	10	1.166e-4	1	NC	5	NC	1
206			min	-.003	2	-.02	4	0	1	5.547e-6	15	5268.617	4	NC	1
207		9	max	.004	3	-.005	15	0	1	1.328e-4	1	NC	5	NC	1
208			min	-.004	2	-.021	4	0	3	6.316e-6	15	4922.233	4	NC	1
209		10	max	.005	3	-.005	15	0	1	1.49e-4	1	NC	5	NC	1
210			min	-.004	2	-.022	4	0	12	7.085e-6	15	4757.377	4	NC	1
211		11	max	.005	3	-.005	15	0	1	1.651e-4	1	NC	5	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.005	2	-.022	4	0	15	7.854e-6	15	4749.848	4	NC	1
213		max	.006	3	-.005	15	0	1	1.813e-4	1	NC	5	NC	1
214		min	-.005	2	-.021	4	0	15	8.623e-6	15	4902.042	4	NC	1
215		max	.006	3	-.005	15	.001	1	1.975e-4	1	NC	5	NC	1
216		min	-.006	2	-.02	4	0	15	9.392e-6	15	5245.067	4	NC	1
217		max	.007	3	-.004	15	.002	1	2.137e-4	1	NC	5	NC	1
218		min	-.006	2	-.018	4	0	15	1.016e-5	15	5854.749	4	NC	1
219		max	.008	3	-.004	15	.002	1	2.299e-4	1	NC	3	NC	1
220		min	-.006	2	-.015	4	0	15	1.093e-5	15	6898.697	4	NC	1
221		max	.008	3	-.003	15	.003	1	2.46e-4	1	NC	1	NC	1
222		min	-.007	2	-.012	4	0	15	1.17e-5	15	8782.493	4	NC	1
223		max	.009	3	-.002	15	.004	1	2.622e-4	1	NC	1	NC	1
224		min	-.007	2	-.009	4	0	15	1.247e-5	15	NC	1	NC	1
225		max	.009	3	-.001	15	.005	1	2.784e-4	1	NC	1	NC	1
226		min	-.008	2	-.005	3	0	15	1.324e-5	15	NC	1	NC	1
227		max	.01	3	0	10	.006	1	2.946e-4	1	NC	1	NC	1
228		min	-.008	2	-.002	3	0	15	1.401e-5	15	NC	1	NC	1
229	M4	max	.002	1	.008	2	0	15	1.132e-4	1	NC	1	NC	2
230		min	0	3	-.01	3	-.006	1	5.419e-6	15	NC	1	4190.99	1
231		max	.002	1	.008	2	0	15	1.132e-4	1	NC	1	NC	2
232		min	0	3	-.009	3	-.005	1	5.419e-6	15	NC	1	4546.859	1
233		max	.002	1	.007	2	0	15	1.132e-4	1	NC	1	NC	2
234		min	0	3	-.009	3	-.005	1	5.419e-6	15	NC	1	4971.064	1
235		max	.002	1	.007	2	0	15	1.132e-4	1	NC	1	NC	2
236		min	0	3	-.008	3	-.005	1	5.419e-6	15	NC	1	5481.237	1
237		max	.002	1	.006	2	0	15	1.132e-4	1	NC	1	NC	2
238		min	0	3	-.008	3	-.004	1	5.419e-6	15	NC	1	6101.353	1
239		max	.002	1	.006	2	0	15	1.132e-4	1	NC	1	NC	2
240		min	0	3	-.007	3	-.004	1	5.419e-6	15	NC	1	6864.66	1
241		max	.001	1	.005	2	0	15	1.132e-4	1	NC	1	NC	2
242		min	0	3	-.007	3	-.003	1	5.419e-6	15	NC	1	7818.353	1
243		max	.001	1	.005	2	0	15	1.132e-4	1	NC	1	NC	2
244		min	0	3	-.006	3	-.003	1	5.419e-6	15	NC	1	9031.252	1
245		max	.001	1	.004	2	0	15	1.132e-4	1	NC	1	NC	1
246		min	0	3	-.006	3	-.002	1	5.419e-6	15	NC	1	NC	1
247		max	.001	1	.004	2	0	15	1.132e-4	1	NC	1	NC	1
248		min	0	3	-.005	3	-.002	1	5.419e-6	15	NC	1	NC	1
249		max	0	1	.004	2	0	15	1.132e-4	1	NC	1	NC	1
250		min	0	3	-.004	3	-.002	1	5.419e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	1.132e-4	1	NC	1	NC	1
252		min	0	3	-.004	3	-.001	1	5.419e-6	15	NC	1	NC	1
253		max	0	1	.003	2	0	15	1.132e-4	1	NC	1	NC	1
254		min	0	3	-.003	3	0	1	5.419e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	1.132e-4	1	NC	1	NC	1
256		min	0	3	-.003	3	0	1	5.419e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	1.132e-4	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	5.419e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	1.132e-4	1	NC	1	NC	1
260		min	0	3	-.002	3	0	1	5.419e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	1.132e-4	1	NC	1	NC	1
262		min	0	3	-.001	3	0	1	5.419e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	1.132e-4	1	NC	1	NC	1
264		min	0	3	0	3	0	1	5.419e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	1.132e-4	1	NC	1	NC	1
266		min	0	1	0	1	0	1	5.419e-6	15	NC	1	NC	1
267	M6	max	.024	2	.039	2	0	1	0	1	NC	3	NC	1
268		min	-.035	3	-.054	3	0	1	0	1	2010.716	2	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.023	2	.035	2	0	1	0	1	NC	3	NC	1
270		min	-.033	3	-.051	3	0	1	0	1	2207.711	2	NC	1
271	3	max	.021	2	.032	2	0	1	0	1	NC	3	NC	1
272		min	-.031	3	-.048	3	0	1	0	1	2445.44	2	NC	1
273	4	max	.02	2	.028	2	0	1	0	1	NC	3	NC	1
274		min	-.029	3	-.045	3	0	1	0	1	2735.442	2	NC	1
275	5	max	.019	2	.025	2	0	1	0	1	NC	3	NC	1
276		min	-.027	3	-.042	3	0	1	0	1	3093.794	2	NC	1
277	6	max	.017	2	.022	2	0	1	0	1	NC	3	NC	1
278		min	-.025	3	-.039	3	0	1	0	1	3543.433	2	NC	1
279	7	max	.016	2	.019	2	0	1	0	1	NC	3	NC	1
280		min	-.023	3	-.036	3	0	1	0	1	4118.003	2	NC	1
281	8	max	.015	2	.016	2	0	1	0	1	NC	1	NC	1
282		min	-.021	3	-.033	3	0	1	0	1	4868.489	2	NC	1
283	9	max	.013	2	.013	2	0	1	0	1	NC	1	NC	1
284		min	-.019	3	-.03	3	0	1	0	1	5875.244	2	NC	1
285	10	max	.012	2	.011	2	0	1	0	1	NC	1	NC	1
286		min	-.017	3	-.027	3	0	1	0	1	7271.059	2	NC	1
287	11	max	.011	2	.008	2	0	1	0	1	NC	1	NC	1
288		min	-.015	3	-.024	3	0	1	0	1	9288.704	2	NC	1
289	12	max	.009	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.014	3	-.021	3	0	1	0	1	NC	1	NC	1
291	13	max	.008	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.012	3	-.018	3	0	1	0	1	NC	1	NC	1
293	14	max	.007	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.01	3	-.015	3	0	1	0	1	NC	1	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.008	3	-.012	3	0	1	0	1	NC	1	NC	1
297	16	max	.004	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.006	3	-.009	3	0	1	0	1	NC	1	NC	1
299	17	max	.003	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.002	3	0	15	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.008	3	0	1	0	1	NC	1	NC	1
311	4	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.011	3	0	1	0	1	NC	1	NC	1
313	5	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.006	2	-.014	3	0	1	0	1	7993.887	3	NC	1
315	6	max	.008	3	-.004	15	0	1	0	1	NC	1	NC	1
316		min	-.007	2	-.017	3	0	1	0	1	6748.551	3	NC	1
317	7	max	.009	3	-.004	15	0	1	0	1	NC	2	NC	1
318		min	-.009	2	-.019	3	0	1	0	1	5924.812	4	NC	1
319	8	max	.011	3	-.005	15	0	1	0	1	NC	2	NC	1
320		min	-.01	2	-.021	3	0	1	0	1	5325.641	4	NC	1
321	9	max	.013	3	-.005	15	0	1	0	1	NC	5	NC	1
322		min	-.012	2	-.022	3	0	1	0	1	4972.127	4	NC	1
323	10	max	.014	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.013	2	-.023	3	0	1	0	1	4802.856	4	NC	1
325	11	max	.016	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.015	2	-.023	3	0	1	0	1	4792.944	4	NC	1
327		12	max	.017	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.016	2	-.022	3	0	1	0	1	4944.506	4	NC	1
329		13	max	.019	3	-.005	15	0	1	0	1	NC	5	NC	1
330			min	-.018	2	-.021	3	0	1	0	1	5288.692	4	NC	1
331		14	max	.02	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.019	2	-.02	3	0	1	0	1	5901.763	4	NC	1
333		15	max	.022	3	-.004	15	0	1	0	1	NC	1	NC	1
334			min	-.021	2	-.018	3	0	1	0	1	6952.476	4	NC	1
335		16	max	.023	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.022	2	-.015	3	0	1	0	1	8849.336	4	NC	1
337		17	max	.025	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.024	2	-.013	3	0	1	0	1	NC	1	NC	1
339		18	max	.027	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.025	2	-.01	3	0	1	0	1	NC	1	NC	1
341		19	max	.028	3	0	10	0	1	0	1	NC	1	NC	1
342			min	-.027	2	-.007	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	2	.026	2	0	1	0	1	NC	1	NC	1
344			min	0	3	-.029	3	0	1	0	1	NC	1	NC	1
345		2	max	.005	2	.025	2	0	1	0	1	NC	1	NC	1
346			min	0	3	-.027	3	0	1	0	1	NC	1	NC	1
347		3	max	.005	2	.023	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.026	3	0	1	0	1	NC	1	NC	1
349		4	max	.005	2	.022	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.024	3	0	1	0	1	NC	1	NC	1
351		5	max	.005	2	.02	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.023	3	0	1	0	1	NC	1	NC	1
353		6	max	.004	2	.019	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.021	3	0	1	0	1	NC	1	NC	1
355		7	max	.004	2	.017	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.019	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	2	.016	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
359		9	max	.003	2	.014	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	2	.013	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.012	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	2	.01	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	2	.009	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.007	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
371		15	max	.001	2	.006	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	0	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	2	.012	2	0	15	1.387e-4	1	NC	1	NC	1
382			min	-.011	3	-.018	3	-.006	1	6.696e-6	15	6404.038	2	NC	1



Envelope Member Section Deflections (Continued)

	Member	Sec	x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
440			min	-0.005	2	-.022	4	0	1	-1.651e-4	1	4749.848	4	NC	1
441		12	max	.006	3	-.005	15	0	15	-8.623e-6	15	NC	5	NC	1
442			min	-0.005	2	-.021	4	0	1	-1.813e-4	1	4902.042	4	NC	1
443		13	max	.006	3	-.005	15	0	15	-9.392e-6	15	NC	5	NC	1
444			min	-.006	2	-.02	4	-.001	1	-1.975e-4	1	5245.067	4	NC	1
445		14	max	.007	3	-.004	15	0	15	-1.016e-5	15	NC	5	NC	1
446			min	-.006	2	-.018	4	-.002	1	-2.137e-4	1	5854.749	4	NC	1
447		15	max	.008	3	-.004	15	0	15	-1.093e-5	15	NC	3	NC	1
448			min	-0.006	2	-.015	4	-.002	1	-2.299e-4	1	6898.697	4	NC	1
449		16	max	.008	3	-.003	15	0	15	-1.17e-5	15	NC	1	NC	1
450			min	-.007	2	-.012	4	-.003	1	-2.46e-4	1	8782.493	4	NC	1
451		17	max	.009	3	-.002	15	0	15	-1.247e-5	15	NC	1	NC	1
452			min	-.007	2	-.009	4	-.004	1	-2.622e-4	1	NC	1	NC	1
453		18	max	.009	3	-.001	15	0	15	-1.324e-5	15	NC	1	NC	1
454			min	-.008	2	-.005	3	-.005	1	-2.784e-4	1	NC	1	NC	1
455		19	max	.01	3	0	10	0	15	-1.401e-5	15	NC	1	NC	1
456			min	-.008	2	-.002	3	-.006	1	-2.946e-4	1	NC	1	NC	1
457	M12	1	max	.002	1	.008	2	.006	1	-5.419e-6	15	NC	1	NC	2
458			min	0	3	-.01	3	0	15	-1.132e-4	1	NC	1	4190.99	1
459		2	max	.002	1	.008	2	.005	1	-5.419e-6	15	NC	1	NC	2
460			min	0	3	-.009	3	0	15	-1.132e-4	1	NC	1	4546.859	1
461		3	max	.002	1	.007	2	.005	1	-5.419e-6	15	NC	1	NC	2
462			min	0	3	-.009	3	0	15	-1.132e-4	1	NC	1	4971.064	1
463		4	max	.002	1	.007	2	.005	1	-5.419e-6	15	NC	1	NC	2
464			min	0	3	-.008	3	0	15	-1.132e-4	1	NC	1	5481.237	1
465		5	max	.002	1	.006	2	.004	1	-5.419e-6	15	NC	1	NC	2
466			min	0	3	-.008	3	0	15	-1.132e-4	1	NC	1	6101.353	1
467		6	max	.002	1	.006	2	.004	1	-5.419e-6	15	NC	1	NC	2
468			min	0	3	-.007	3	0	15	-1.132e-4	1	NC	1	6864.66	1
469		7	max	.001	1	.005	2	.003	1	-5.419e-6	15	NC	1	NC	2
470			min	0	3	-.007	3	0	15	-1.132e-4	1	NC	1	7818.353	1
471		8	max	.001	1	.005	2	.003	1	-5.419e-6	15	NC	1	NC	2
472			min	0	3	-.006	3	0	15	-1.132e-4	1	NC	1	9031.252	1
473		9	max	.001	1	.004	2	.002	1	-5.419e-6	15	NC	1	NC	1
474			min	0	3	-.006	3	0	15	-1.132e-4	1	NC	1	NC	1
475		10	max	.001	1	.004	2	.002	1	-5.419e-6	15	NC	1	NC	1
476			min	0	3	-.005	3	0	15	-1.132e-4	1	NC	1	NC	1
477		11	max	0	1	.004	2	.002	1	-5.419e-6	15	NC	1	NC	1
478			min	0	3	-.004	3	0	15	-1.132e-4	1	NC	1	NC	1
479		12	max	0	1	.003	2	.001	1	-5.419e-6	15	NC	1	NC	1
480			min	0	3	-.004	3	0	15	-1.132e-4	1	NC	1	NC	1
481		13	max	0	1	.003	2	0	1	-5.419e-6	15	NC	1	NC	1
482			min	0	3	-.003	3	0	15	-1.132e-4	1	NC	1	NC	1
483		14	max	0	1	.002	2	0	1	-5.419e-6	15	NC	1	NC	1
484			min	0	3	-.003	3	0	15	-1.132e-4	1	NC	1	NC	1
485		15	max	0	1	.002	2	0	1	-5.419e-6	15	NC	1	NC	1
486			min	0	3	-.002	3	0	15	-1.132e-4	1	NC	1	NC	1
487		16	max	0	1	.001	2	0	1	-5.419e-6	15	NC	1	NC	1
488			min	0	3	-.002	3	0	15	-1.132e-4	1	NC	1	NC	1
489		17	max	0	1	0	2	0	1	-5.419e-6	15	NC	1	NC	1
490			min	0	3	-.001	3	0	15	-1.132e-4	1	NC	1	NC	1
491		18	max	0	1	0	2	0	1	-5.419e-6	15	NC	1	NC	1
492			min	0	3	0	3	0	15	-1.132e-4	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-5.419e-6	15	NC	1	NC	1
494			min	0	1	0	1	0	1	-1.132e-4	1	NC	1	NC	1
495	M1	1	max	.012	3	.224	2	0	1	5.81e-3	2	NC	1	NC	1
496			min	-.008	2	-.072	3	0	15	-1.522e-2	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.012	3	.108	2	0	15	2.85e-3	2	NC	5	NC	1
498			min	-.008	2	-.033	3	-.005	1	-7.556e-3	3	1172.471	2	NC	1
499		3	max	.012	3	.019	3	0	15	2.258e-5	10	NC	5	NC	1
500			min	-.008	2	-.015	2	-.006	1	-1.111e-4	1	569.071	2	NC	1
501		4	max	.012	3	.096	3	0	15	3.743e-3	2	NC	5	NC	1
502			min	-.008	2	-.149	2	-.006	1	-3.925e-3	3	363.342	2	NC	1
503		5	max	.012	3	.188	3	0	15	7.48e-3	2	NC	15	NC	1
504			min	-.007	2	-.288	2	-.004	1	-7.751e-3	3	264.683	2	NC	1
505		6	max	.011	3	.286	3	0	15	1.122e-2	2	9182.461	15	NC	1
506			min	-.007	2	-.422	2	-.002	1	-1.158e-2	3	209.961	2	NC	1
507		7	max	.011	3	.378	3	0	1	1.495e-2	2	7775.299	15	NC	1
508			min	-.007	2	-.54	2	0	3	-1.54e-2	3	177.483	2	NC	1
509		8	max	.011	3	.455	3	0	1	1.869e-2	2	6940.381	15	NC	1
510			min	-.007	2	-.633	2	0	15	-1.923e-2	3	158.2	2	NC	1
511		9	max	.011	3	.504	3	0	15	2.091e-2	2	6502.367	15	NC	1
512			min	-.007	2	-.692	2	0	1	-1.987e-2	3	148.127	2	NC	1
513		10	max	.01	3	.522	3	0	1	2.212e-2	2	6368.106	15	NC	1
514			min	-.007	2	-.712	2	0	15	-1.839e-2	3	145.185	2	NC	1
515		11	max	.01	3	.51	3	0	1	2.333e-2	2	6501.826	15	NC	1
516			min	-.007	2	-.692	2	0	15	-1.69e-2	3	148.688	2	NC	1
517		12	max	.01	3	.468	3	0	15	2.228e-2	2	6939.206	15	NC	1
518			min	-.006	2	-.63	2	0	1	-1.482e-2	3	159.829	2	NC	1
519		13	max	.01	3	.399	3	0	10	1.786e-2	2	7773.207	15	NC	1
520			min	-.006	2	-.532	2	0	1	-1.186e-2	3	181.293	2	NC	1
521		14	max	.009	3	.311	3	.001	1	1.345e-2	2	9178.891	15	NC	1
522			min	-.006	2	-.41	2	0	15	-8.895e-3	3	217.875	2	NC	1
523		15	max	.009	3	.212	3	.004	1	9.035e-3	2	NC	15	NC	1
524			min	-.006	2	-.274	2	0	15	-5.931e-3	3	280.559	2	NC	1
525		16	max	.009	3	.108	3	.005	1	4.621e-3	2	NC	5	NC	1
526			min	-.006	2	-.136	2	0	15	-2.967e-3	3	395.894	2	NC	1
527		17	max	.009	3	.007	3	.006	1	4.041e-4	1	NC	5	NC	1
528			min	-.006	2	-.008	2	0	15	-3.879e-6	3	640.546	2	NC	1
529		18	max	.009	3	.101	2	.004	1	5.279e-3	2	NC	5	NC	1
530			min	-.006	2	-.086	3	0	15	-1.75e-3	3	1351.428	2	NC	1
531		19	max	.009	3	.199	2	0	15	1.052e-2	2	NC	1	NC	1
532			min	-.006	2	-.173	3	0	1	-3.581e-3	3	NC	1	NC	1
533	M5	1	max	.036	3	.339	2	0	1	0	1	NC	1	NC	1
534			min	-.025	2	-.007	3	0	1	0	1	NC	1	NC	1
535		2	max	.036	3	.161	2	0	1	0	1	NC	5	NC	1
536			min	-.025	2	.002	3	0	1	0	1	773.894	2	NC	1
537		3	max	.035	3	.056	3	0	1	0	1	NC	5	NC	1
538			min	-.025	2	-.042	2	0	1	0	1	359.963	2	NC	1
539		4	max	.035	3	.188	3	0	1	0	1	NC	15	NC	1
540			min	-.025	2	-.29	2	0	1	0	1	217.308	2	NC	1
541		5	max	.034	3	.377	3	0	1	0	1	7389.803	15	NC	1
542			min	-.024	2	-.564	2	0	1	0	1	151.162	2	NC	1
543		6	max	.033	3	.593	3	0	1	0	1	5647.436	15	NC	1
544			min	-.024	2	-.839	2	0	1	0	1	115.817	2	NC	1
545		7	max	.032	3	.805	3	0	1	0	1	4648.995	15	NC	1
546			min	-.024	2	-1.09	2	0	1	0	1	95.476	2	NC	1
547		8	max	.032	3	.984	3	0	1	0	1	4071.967	15	NC	1
548			min	-.023	2	-1.292	2	0	1	0	1	83.682	2	NC	1
549		9	max	.031	3	1.099	3	0	1	0	1	3777.007	15	NC	1
550			min	-.023	2	-1.42	2	0	1	0	1	77.641	2	NC	1
551		10	max	.03	3	1.141	3	0	1	0	1	3688.247	15	NC	1
552			min	-.022	2	-1.464	2	0	1	0	1	75.883	2	NC	1
553		11	max	.029	3	1.112	3	0	1	0	1	3777.371	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.022	2	-1.421	2	0	1	0	1	77.974	2	NC	1
555		12	max	.029	3	1.014	3	0	1	0	1	4072.813	15	NC	1
556			min	-.021	2	-1.287	2	0	1	0	1	84.798	2	NC	1
557		13	max	.028	3	.856	3	0	1	0	1	4650.65	15	NC	1
558			min	-.021	2	-1.072	2	0	1	0	1	98.431	2	NC	1
559		14	max	.027	3	.658	3	0	1	0	1	5650.572	15	NC	1
560			min	-.021	2	-.807	2	0	1	0	1	122.648	2	NC	1
561		15	max	.026	3	.44	3	0	1	0	1	7395.884	15	NC	1
562			min	-.02	2	-.523	2	0	1	0	1	166.528	2	NC	1
563		16	max	.026	3	.22	3	0	1	0	1	NC	15	NC	1
564			min	-.02	2	-.251	2	0	1	0	1	253.508	2	NC	1
565		17	max	.025	3	.018	3	0	1	0	1	NC	5	NC	1
566			min	-.02	2	-.022	2	0	1	0	1	453.876	2	NC	1
567		18	max	.025	3	.141	2	0	1	0	1	NC	5	NC	1
568			min	-.02	2	-.15	3	0	1	0	1	1014.192	3	NC	1
569		19	max	.025	3	.266	2	0	1	0	1	NC	1	NC	1
570			min	-.02	2	-.298	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.012	3	.224	2	0	15	1.522e-2	3	NC	1	NC	1
572			min	-.008	2	-.072	3	0	1	-5.81e-3	2	NC	1	NC	1
573		2	max	.012	3	.108	2	.005	1	7.556e-3	3	NC	5	NC	1
574			min	-.008	2	-.033	3	0	15	-2.85e-3	2	1172.471	2	NC	1
575		3	max	.012	3	.019	3	.006	1	1.111e-4	1	NC	5	NC	1
576			min	-.008	2	-.015	2	0	15	-2.258e-5	10	569.071	2	NC	1
577		4	max	.012	3	.096	3	.006	1	3.925e-3	3	NC	5	NC	1
578			min	-.008	2	-.149	2	0	15	-3.743e-3	2	363.342	2	NC	1
579		5	max	.012	3	.188	3	.004	1	7.751e-3	3	NC	15	NC	1
580			min	-.007	2	-.288	2	0	15	-7.48e-3	2	264.683	2	NC	1
581		6	max	.011	3	.286	3	.002	1	1.158e-2	3	9182.461	15	NC	1
582			min	-.007	2	-.422	2	0	15	-1.122e-2	2	209.961	2	NC	1
583		7	max	.011	3	.378	3	0	3	1.54e-2	3	7775.299	15	NC	1
584			min	-.007	2	-.54	2	0	1	-1.495e-2	2	177.483	2	NC	1
585		8	max	.011	3	.455	3	0	15	1.923e-2	3	6940.381	15	NC	1
586			min	-.007	2	-.633	2	0	1	-1.869e-2	2	158.2	2	NC	1
587		9	max	.011	3	.504	3	0	1	1.987e-2	3	6502.367	15	NC	1
588			min	-.007	2	-.692	2	0	15	-2.091e-2	2	148.127	2	NC	1
589		10	max	.01	3	.522	3	0	15	1.839e-2	3	6368.106	15	NC	1
590			min	-.007	2	-.712	2	0	1	-2.212e-2	2	145.185	2	NC	1
591		11	max	.01	3	.51	3	0	15	1.69e-2	3	6501.826	15	NC	1
592			min	-.007	2	-.692	2	0	1	-2.333e-2	2	148.688	2	NC	1
593		12	max	.01	3	.468	3	0	1	1.482e-2	3	6939.206	15	NC	1
594			min	-.006	2	-.63	2	0	15	-2.228e-2	2	159.829	2	NC	1
595		13	max	.01	3	.399	3	0	1	1.186e-2	3	7773.207	15	NC	1
596			min	-.006	2	-.532	2	0	10	-1.786e-2	2	181.293	2	NC	1
597		14	max	.009	3	.311	3	0	15	8.895e-3	3	9178.891	15	NC	1
598			min	-.006	2	-.41	2	-.001	1	-1.345e-2	2	217.875	2	NC	1
599		15	max	.009	3	.212	3	0	15	5.931e-3	3	NC	15	NC	1
600			min	-.006	2	-.274	2	-.004	1	-9.035e-3	2	280.559	2	NC	1
601		16	max	.009	3	.108	3	0	15	2.967e-3	3	NC	5	NC	1
602			min	-.006	2	-.136	2	-.005	1	-4.621e-3	2	395.894	2	NC	1
603		17	max	.009	3	.007	3	0	15	3.879e-6	3	NC	5	NC	1
604			min	-.006	2	-.008	2	-.006	1	-4.041e-4	1	640.546	2	NC	1
605		18	max	.009	3	.101	2	0	15	1.75e-3	3	NC	5	NC	1
606			min	-.006	2	-.086	3	-.004	1	-5.279e-3	2	1351.428	2	NC	1
607		19	max	.009	3	.199	2	0	1	3.581e-3	3	NC	1	NC	1
608			min	-.006	2	-.173	3	0	15	-1.052e-2	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

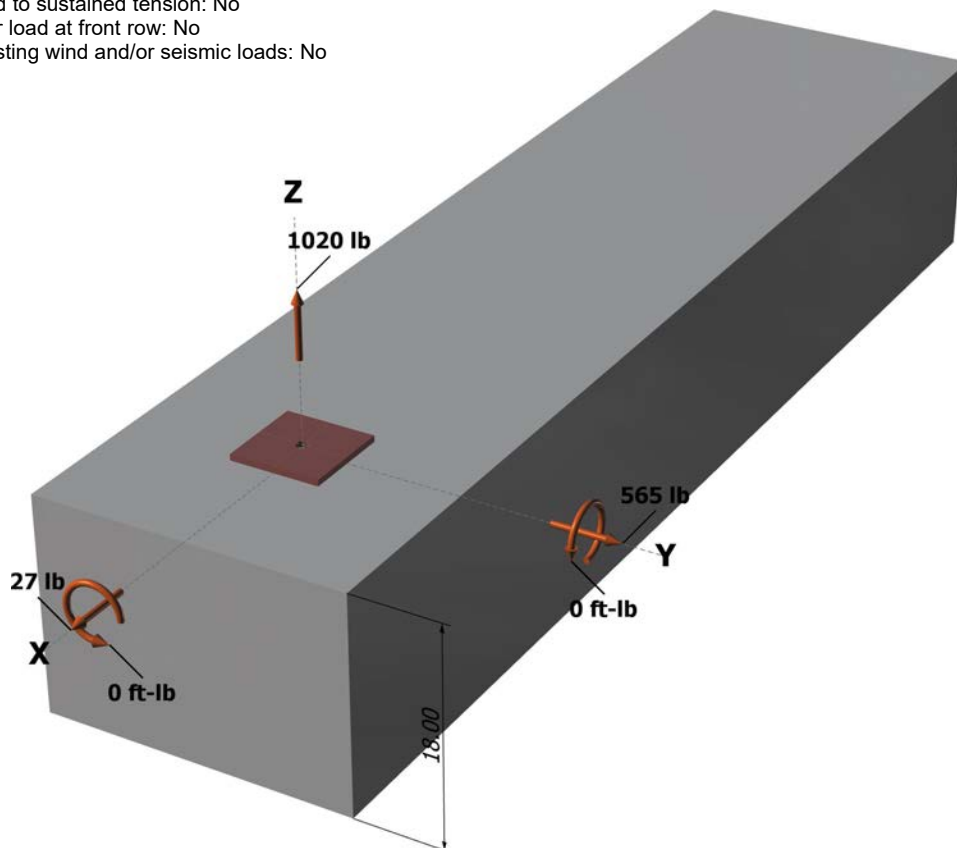
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cby} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cby} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

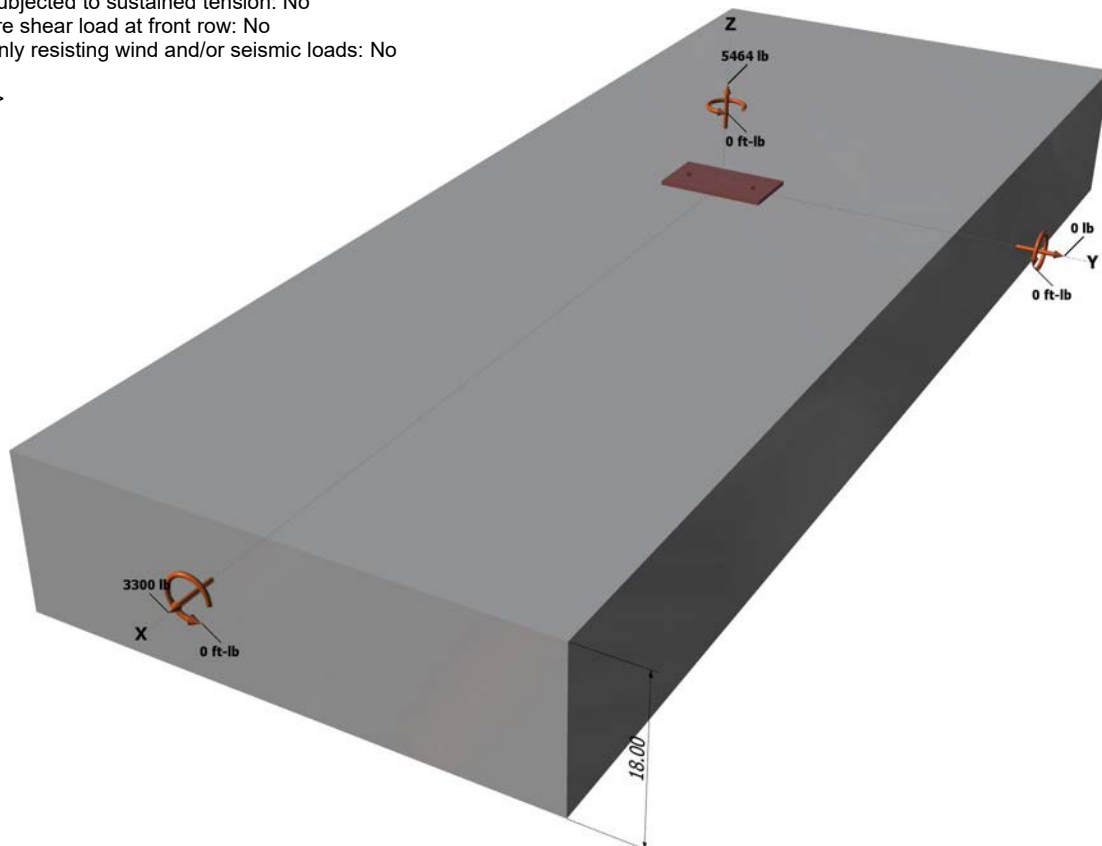
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

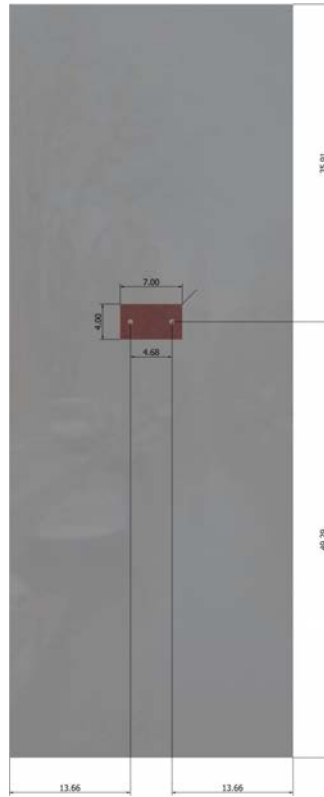
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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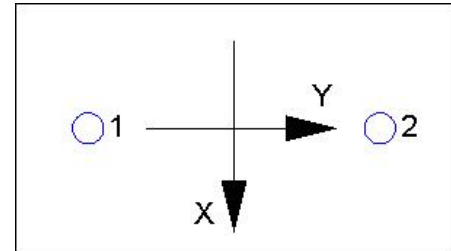
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 5464
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cp} = 20601$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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- Designer must exercise own judgement to determine if this design is suitable.
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