

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.985 k
$M_y \text{ allowable}$ =	0.423 k-ft
$M_z \text{ allowable}$ =	0.423 k-ft
$P_n \text{ allowable}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

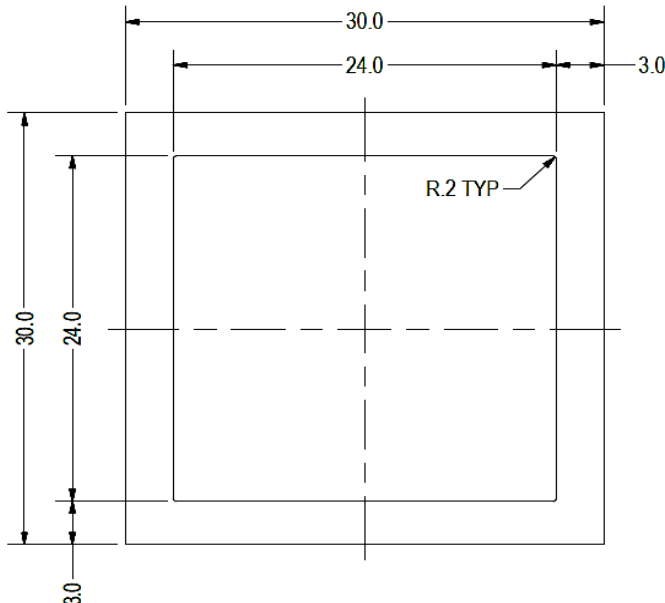
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.329 k
$M_y \text{ allowable}$ =	0.404 k-ft
$M_z \text{ allowable}$ =	0.404 k-ft
$P_n \text{ allowable}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

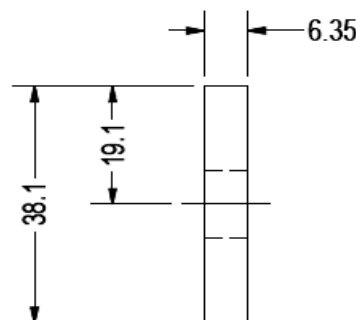
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.767 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	11%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.168 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

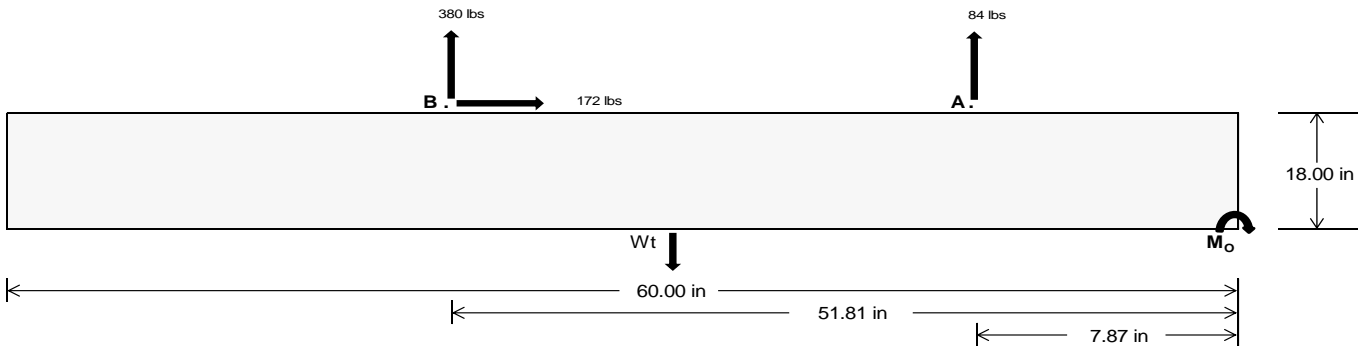
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	354.09	1583.39	k
Compressive Load =	1279.92	1057.33	k
Lateral Load =	22.87	715.03	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23453.1$ in-lbs
Resisting Force Required = 781.77 lbs
S.F. = 1.67
Weight Required = 1302.95 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 171.87 lbs
Friction = 0.4
Weight Required = 429.67 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 171.87 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	409 lbs	409 lbs	409 lbs	409 lbs	506 lbs	506 lbs	506 lbs	506 lbs	655 lbs	655 lbs	655 lbs	655 lbs	-169 lbs	-169 lbs	-169 lbs	-169 lbs
F_B	293 lbs	293 lbs	293 lbs	293 lbs	443 lbs	443 lbs	443 lbs	443 lbs	529 lbs	529 lbs	529 lbs	529 lbs	-760 lbs	-760 lbs	-760 lbs	-760 lbs
F_V	29 lbs	29 lbs	29 lbs	29 lbs	302 lbs	302 lbs	302 lbs	302 lbs	246 lbs	246 lbs	246 lbs	246 lbs	-344 lbs	-344 lbs	-344 lbs	-344 lbs
P_{total}	2696 lbs	2787 lbs	2877 lbs	2968 lbs	2942 lbs	3033 lbs	3124 lbs	3214 lbs	3178 lbs	3268 lbs	3359 lbs	3449 lbs	267 lbs	322 lbs	376 lbs	430 lbs
M	265 lbs-ft	265 lbs-ft	265 lbs-ft	265 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft	617 lbs-ft	617 lbs-ft	617 lbs-ft	617 lbs-ft	555 lbs-ft	555 lbs-ft	555 lbs-ft	555 lbs-ft
e	0.10 ft	0.10 ft	0.09 ft	0.09 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.08 ft	1.73 ft	1.48 ft	1.29 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	259.4 psf	257.6 psf	255.9 psf	254.4 psf	244.8 psf	243.6 psf	242.5 psf	241.5 psf	265.9 psf	263.8 psf	261.9 psf	260.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	328.8 psf	324.0 psf	319.5 psf	315.5 psf	397.1 psf	389.3 psf	382.2 psf	375.6 psf	427.4 psf	418.3 psf	409.9 psf	402.2 psf	230.0 psf	144.6 psf	122.5 psf	113.8 psf

Maximum Bearing Pressure = 427 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

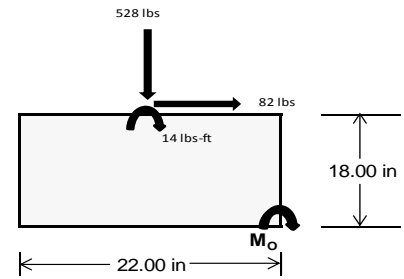
Overturning Check

$M_o = 347.7 \text{ ft-lbs}$
 Resisting Force Required = 379.29 lbs
 S.F. = 1.67
 Weight Required = 632.15 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	109 lbs	75 lbs	54 lbs	237 lbs	528 lbs	195 lbs	71 lbs	-20 lbs	19 lbs
F_v	13 lbs	109 lbs	13 lbs	9 lbs	82 lbs	10 lbs	13 lbs	109 lbs	13 lbs
P_{total}	2577 lbs	2543 lbs	2523 lbs	2586 lbs	2878 lbs	2545 lbs	793 lbs	701 lbs	741 lbs
M	37 lbs-ft	182 lbs-ft	38 lbs-ft	26 lbs-ft	137 lbs-ft	29 lbs-ft	37 lbs-ft	182 lbs-ft	38 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.26 ft	0.05 ft
$L/6$	0.31 ft	1.69 ft	1.80 ft	1.81 ft	1.74 ft	1.81 ft	1.74 ft	1.31 ft	1.73 ft
f_{min}	268.1 sqft	212.4 sqft	261.8 sqft	272.9 sqft	265.2 sqft	267.3 sqft	73.4 sqft	11.5 sqft	67.4 sqft
f_{max}	294.2 psf	342.5 psf	288.6 psf	291.4 psf	362.8 psf	287.9 psf	99.6 psf	141.5 psf	94.2 psf



Maximum Bearing Pressure = 363 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

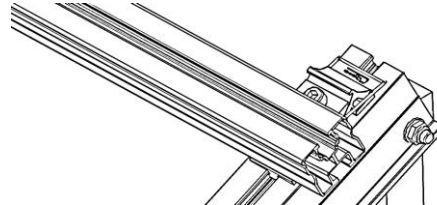
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.722 k
Allowable Uplift =	1.214 k
Utilization =	<u>59%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.092 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.985 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Diagonal Strut

Maximum Axial Load =	0.329 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.126 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.168 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.057 in
	<u>0.057 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$132.801$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$137.906$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.261 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &21.5928 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.459 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				2		
2	Dead Load, Min	DL		-1				2		
3	Snow Load	SL						2		
4	Wind Load - Pressure	WL						2		
5	Wind Load - Suction	WL						2		
6	Seismic - Lateral	EL			.8			4		

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M16	Y	-8.366	-8.366	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M16	Y	-4.45	-4.45	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-57.498	-57.498	0	0
2	M16	Y	-57.498	-57.498	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-66.204	-66.204	0	0
2	M16	y	-104.034	-104.034	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	133.668	133.668	0	0
2	M16	y	63.051	63.051	0	0

Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M16	Z	6.693	6.693	0	0
3	M13	Z	0	0	0	0
4	M16	Z	0	0	0	0

Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes	Y			1	1.2	3	1.6	4	.8									
2	LRFD 1.2D + 1.6W + 0.5S	Yes	Y			1	1.2	3	.5	4	1.6									
3	LRFD 0.9D + 1.6W	Yes	Y			2	.9					5	1.6							
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y			1	1.54	3	.2			6	1.3							
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y			1	.56					6	1.3							
6	LATERAL - LRFD 1.54D + 1.25E	Yes	Y			1	1.54	3	.2			6	1.25							
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y			1	.56					6	1.25							
8																				
9	ASD 1.0D + 1.0S	Yes	Y			1	1	3	1											



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 20° 120mph 30psf 4.25ft 7-05Page 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	243.095	1	.081	2	.162	1	0	10	0	4	0	15
30		min	-358.755	3	.014	12	-.45	5	0	4	0	3	0	6
31	16	max	243.202	1	.048	2	.162	1	0	10	0	4	0	15
32		min	-358.675	3	-.005	3	-.547	5	0	4	0	3	0	6
33	17	max	243.308	1	.016	2	.162	1	0	10	0	4	0	15
34		min	-358.595	3	-.029	3	-.643	5	0	4	0	3	0	6
35	18	max	243.415	1	-.015	10	.162	1	0	10	0	1	0	15
36		min	-358.515	3	-.058	4	-.74	5	0	4	0	3	0	6
37	19	max	243.521	1	-.025	15	.162	1	0	10	0	1	0	15
38		min	-358.435	3	-.099	4	-.836	5	0	4	0	3	0	6
39	M3	1	max	95.89	2	1.795	6	0	10	0	5	0	4	6
40		min	-86.88	3	.421	15	-1.349	4	0	1	0	10	0	15
41	2	max	95.822	2	1.618	6	0	10	0	5	0	1	0	6
42		min	-86.931	3	.379	15	-1.216	4	0	1	0	10	0	15
43	3	max	95.754	2	1.44	6	0	10	0	5	0	1	0	2
44		min	-86.982	3	.337	15	-1.082	4	0	1	0	5	0	3
45	4	max	95.686	2	1.262	6	0	10	0	5	0	1	0	15
46		min	-87.033	3	.296	15	-.948	4	0	1	0	5	0	4
47	5	max	95.618	2	1.085	6	0	10	0	5	0	1	0	15
48		min	-87.083	3	.254	15	-.815	4	0	1	0	5	0	4
49	6	max	95.55	2	.907	6	0	10	0	5	0	1	0	15
50		min	-87.134	3	.212	15	-.681	4	0	1	0	5	0	4
51	7	max	95.482	2	.73	6	0	10	0	5	0	1	0	15
52		min	-87.185	3	.17	15	-.548	4	0	1	0	5	0	4
53	8	max	95.415	2	.552	6	0	10	0	5	0	1	0	15
54		min	-87.236	3	.129	15	-.414	4	0	1	0	5	0	4
55	9	max	95.347	2	.374	6	0	10	0	5	0	1	0	15
56		min	-87.287	3	.087	15	-.28	4	0	1	0	5	-.001	4
57	10	max	95.279	2	.197	6	0	10	0	5	0	1	0	15
58		min	-87.338	3	.045	15	-.173	1	0	1	0	5	-.001	4
59	11	max	95.211	2	.036	2	.024	5	0	5	0	1	0	15
60		min	-87.389	3	-.003	3	-.173	1	0	1	0	5	-.001	4
61	12	max	95.143	2	-.039	15	.158	5	0	5	0	1	0	15
62		min	-87.44	3	-.159	4	-.173	1	0	1	0	5	-.001	4
63	13	max	95.075	2	-.08	15	.291	5	0	5	0	1	0	15
64		min	-87.491	3	-.337	4	-.173	1	0	1	0	5	-.001	4
65	14	max	95.007	2	-.122	15	.425	5	0	5	0	1	0	15
66		min	-87.541	3	-.514	4	-.173	1	0	1	0	5	-.001	4
67	15	max	94.94	2	-.164	15	.559	5	0	5	0	10	0	15
68		min	-87.592	3	-.692	4	-.173	1	0	1	0	5	0	4
69	16	max	94.872	2	-.206	15	.692	5	0	5	0	10	0	15
70		min	-87.643	3	-.87	4	-.173	1	0	1	0	4	0	4
71	17	max	94.804	2	-.247	15	.826	5	0	5	0	10	0	15
72		min	-87.694	3	-1.047	4	-.173	1	0	1	0	4	0	4
73	18	max	94.736	2	-.289	15	.96	5	0	5	0	10	0	15
74		min	-87.745	3	-1.225	4	-.173	1	0	1	0	4	0	4
75	19	max	94.668	2	-.331	15	1.093	5	0	5	0	5	0	1
76		min	-87.796	3	-1.402	4	-.173	1	0	1	0	1	0	1
77	M4	1	max	334.667	1	0	1	.008	10	0	1	0	5	1
78		min	-76.392	3	0	1	-16.408	4	0	1	0	2	0	1
79	2	max	334.732	1	0	1	.008	10	0	1	0	10	0	1
80		min	-76.343	3	0	1	-16.464	4	0	1	-.001	4	0	1
81	3	max	334.796	1	0	1	.008	10	0	1	0	10	0	1
82		min	-76.295	3	0	1	-16.52	4	0	1	-.003	4	0	1
83	4	max	334.861	1	0	1	.008	10	0	1	0	10	0	1
84		min	-76.246	3	0	1	-16.576	4	0	1	-.004	4	0	1
85	5	max	334.926	1	0	1	.008	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-76.198	3	0	1	-16.632	4	0	1	-.006	4	0	1
87		6	max	334.991	1	0	1	.008	10	0	1	0	10	0	1
88			min	-76.149	3	0	1	-16.688	4	0	1	-.007	4	0	1
89		7	max	335.055	1	0	1	.008	10	0	1	0	10	0	1
90			min	-76.101	3	0	1	-16.745	4	0	1	-.009	4	0	1
91		8	max	335.12	1	0	1	.008	10	0	1	0	10	0	1
92			min	-76.052	3	0	1	-16.801	4	0	1	-.01	4	0	1
93		9	max	335.185	1	0	1	.008	10	0	1	0	10	0	1
94			min	-76.003	3	0	1	-16.857	4	0	1	-.012	4	0	1
95		10	max	335.249	1	0	1	.008	10	0	1	0	10	0	1
96			min	-75.955	3	0	1	-16.913	4	0	1	-.013	4	0	1
97		11	max	335.314	1	0	1	.008	10	0	1	0	10	0	1
98			min	-75.906	3	0	1	-16.969	4	0	1	-.015	4	0	1
99		12	max	335.379	1	0	1	.008	10	0	1	0	10	0	1
100			min	-75.858	3	0	1	-17.025	4	0	1	-.016	4	0	1
101		13	max	335.444	1	0	1	.008	10	0	1	0	10	0	1
102			min	-75.809	3	0	1	-17.081	4	0	1	-.018	4	0	1
103		14	max	335.508	1	0	1	.008	10	0	1	0	10	0	1
104			min	-75.761	3	0	1	-17.137	4	0	1	-.019	4	0	1
105		15	max	335.573	1	0	1	.008	10	0	1	0	10	0	1
106			min	-75.712	3	0	1	-17.193	4	0	1	-.021	4	0	1
107		16	max	335.638	1	0	1	.008	10	0	1	0	10	0	1
108			min	-75.664	3	0	1	-17.249	4	0	1	-.023	4	0	1
109		17	max	335.702	1	0	1	.008	10	0	1	0	10	0	1
110			min	-75.615	3	0	1	-17.305	4	0	1	-.024	4	0	1
111		18	max	335.767	1	0	1	.008	10	0	1	0	10	0	1
112			min	-75.567	3	0	1	-17.361	4	0	1	-.026	4	0	1
113		19	max	335.832	1	0	1	.008	10	0	1	0	10	0	1
114			min	-75.518	3	0	1	-17.418	4	0	1	-.027	4	0	1
115	M6	1	max	764.951	1	.632	6	.917	4	0	3	0	3	0	1
116			min	-1126.447	3	.144	15	-.261	3	0	5	0	2	0	1
117		2	max	765.057	1	.591	6	.82	4	0	3	0	4	0	15
118			min	-1126.367	3	.134	15	-.261	3	0	5	0	2	0	6
119		3	max	765.164	1	.55	6	.724	4	0	3	0	4	0	15
120			min	-1126.287	3	.125	15	-.261	3	0	5	0	2	0	6
121		4	max	765.27	1	.508	6	.627	4	0	3	0	4	0	15
122			min	-1126.207	3	.115	15	-.261	3	0	5	0	2	0	6
123		5	max	765.377	1	.467	6	.531	4	0	3	0	4	0	15
124			min	-1126.127	3	.105	15	-.261	3	0	5	0	2	0	6
125		6	max	765.484	1	.426	6	.435	4	0	3	0	4	0	15
126			min	-1126.047	3	.096	15	-.261	3	0	5	0	3	0	6
127		7	max	765.59	1	.39	2	.338	4	0	3	0	4	0	15
128			min	-1125.967	3	.086	15	-.261	3	0	5	0	3	0	6
129		8	max	765.697	1	.358	2	.242	4	0	3	0	4	0	15
130			min	-1125.888	3	.076	15	-.261	3	0	5	0	3	0	6
131		9	max	765.803	1	.326	2	.145	4	0	3	0	4	0	15
132			min	-1125.808	3	.067	15	-.261	3	0	5	0	3	0	6
133		10	max	765.91	1	.294	2	.052	14	0	3	0	4	0	15
134			min	-1125.728	3	.057	15	-.261	3	0	5	0	3	0	6
135		11	max	766.016	1	.262	2	.041	9	0	3	0	4	0	15
136			min	-1125.648	3	.047	15	-.261	3	0	5	0	3	0	6
137		12	max	766.123	1	.23	2	.041	9	0	3	0	4	0	15
138			min	-1125.568	3	.036	12	-.261	3	0	5	0	3	0	2
139		13	max	766.229	1	.197	2	.041	9	0	3	0	4	0	15
140			min	-1125.488	3	.02	12	-.261	3	0	5	0	3	0	2
141		14	max	766.336	1	.165	2	.041	9	0	3	0	4	0	15
142			min	-1125.408	3	.001	3	-.353	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	766.442	1	.133	2	.041	9	0	3	0	4	0	15
144			min	-1125.328	3	-.023	3	-.449	5	0	5	0	3	0	2
145		16	max	766.549	1	.101	2	.041	9	0	3	0	4	0	15
146			min	-1125.248	3	-.047	3	-.545	5	0	5	0	3	0	2
147		17	max	766.656	1	.069	2	.041	9	0	3	0	4	0	15
148			min	-1125.168	3	-.071	3	-.642	5	0	5	0	3	0	2
149		18	max	766.762	1	.037	2	.041	9	0	3	0	4	0	15
150			min	-1125.088	3	-.095	3	-.738	5	0	5	0	3	0	2
151		19	max	766.869	1	.005	2	.041	9	0	3	0	4	0	15
152			min	-1125.009	3	-.119	3	-.835	5	0	5	0	3	0	2
153	M7	1	max	329.142	2	1.806	4	.008	3	0	9	0	4	0	2
154			min	-236.015	3	.428	15	-1.385	4	0	3	0	3	0	12
155		2	max	329.074	2	1.628	4	.008	3	0	9	0	4	0	2
156			min	-236.066	3	.387	15	-1.251	4	0	3	0	3	0	12
157		3	max	329.006	2	1.45	4	.008	3	0	9	0	1	0	2
158			min	-236.117	3	.345	15	-1.118	4	0	3	0	3	0	3
159		4	max	328.939	2	1.273	4	.008	3	0	9	0	1	0	2
160			min	-236.168	3	.303	15	-.984	4	0	3	0	3	0	3
161		5	max	328.871	2	1.095	4	.008	3	0	9	0	1	0	15
162			min	-236.219	3	.261	15	-.85	4	0	3	0	5	0	6
163		6	max	328.803	2	.917	4	.008	3	0	9	0	1	0	15
164			min	-236.27	3	.22	15	-.717	4	0	3	0	5	0	6
165		7	max	328.735	2	.74	4	.008	3	0	9	0	1	0	15
166			min	-236.321	3	.178	15	-.583	4	0	3	0	5	0	6
167		8	max	328.667	2	.562	4	.008	3	0	9	0	1	0	15
168			min	-236.372	3	.136	15	-.45	4	0	3	0	5	0	6
169		9	max	328.599	2	.384	4	.008	3	0	9	0	1	0	15
170			min	-236.423	3	.094	15	-.316	4	0	3	0	5	-.001	6
171		10	max	328.531	2	.212	2	.008	3	0	9	0	1	0	15
172			min	-236.473	3	.047	12	-.182	4	0	3	0	5	-.001	6
173		11	max	328.463	2	.074	2	.008	3	0	9	0	1	0	15
174			min	-236.524	3	-.036	3	-.049	4	0	3	0	5	-.001	6
175		12	max	328.396	2	-.031	15	.086	5	0	9	0	1	0	15
176			min	-236.575	3	-.149	6	-.013	1	0	3	0	5	-.001	6
177		13	max	328.328	2	-.073	15	.22	5	0	9	0	1	0	15
178			min	-236.626	3	-.327	6	-.013	1	0	3	0	5	-.001	6
179		14	max	328.26	2	-.114	15	.354	5	0	9	0	1	0	15
180			min	-236.677	3	-.504	6	-.013	1	0	3	0	5	-.001	6
181		15	max	328.192	2	-.156	15	.487	5	0	9	0	1	0	15
182			min	-236.728	3	-.682	6	-.013	1	0	3	0	5	0	6
183		16	max	328.124	2	-.198	15	.621	5	0	9	0	1	0	15
184			min	-236.779	3	-.86	6	-.013	1	0	3	0	5	0	6
185		17	max	328.056	2	-.24	15	.754	5	0	9	0	1	0	15
186			min	-236.83	3	-1.037	6	-.013	1	0	3	0	5	0	6
187		18	max	327.988	2	-.281	15	.888	5	0	9	0	1	0	15
188			min	-236.881	3	-1.215	6	-.013	1	0	3	0	5	0	6
189		19	max	327.921	2	-.323	15	1.022	5	0	9	0	9	0	1
190			min	-236.931	3	-1.393	6	-.013	1	0	3	0	3	0	1
191	M8	1	max	983.388	1	0	1	.153	9	0	1	0	4	0	1
192			min	-273.253	3	0	1	-16.737	4	0	1	0	3	0	1
193		2	max	983.453	1	0	1	.153	9	0	1	0	9	0	1
194			min	-273.205	3	0	1	-16.793	4	0	1	-.001	4	0	1
195		3	max	983.518	1	0	1	.153	9	0	1	0	9	0	1
196			min	-273.156	3	0	1	-16.849	4	0	1	-.003	4	0	1
197		4	max	983.582	1	0	1	.153	9	0	1	0	9	0	1
198			min	-273.108	3	0	1	-16.905	4	0	1	-.004	4	0	1
199		5	max	983.647	1	0	1	.153	9	0	1	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-273.059	3	0	1	-16.961	4	0	1	-.006	4	0	1
201		6	max	983.712	1	0	1	.153	9	0	1	0	9	0	1
202			min	-273.01	3	0	1	-17.017	4	0	1	-.008	4	0	1
203		7	max	983.777	1	0	1	.153	9	0	1	0	9	0	1
204			min	-272.962	3	0	1	-17.073	4	0	1	-.009	4	0	1
205		8	max	983.841	1	0	1	.153	9	0	1	0	9	0	1
206			min	-272.913	3	0	1	-17.13	4	0	1	-.011	4	0	1
207		9	max	983.906	1	0	1	.153	9	0	1	0	9	0	1
208			min	-272.865	3	0	1	-17.186	4	0	1	-.012	4	0	1
209		10	max	983.971	1	0	1	.153	9	0	1	0	9	0	1
210			min	-272.816	3	0	1	-17.242	4	0	1	-.014	4	0	1
211		11	max	984.035	1	0	1	.153	9	0	1	0	9	0	1
212			min	-272.768	3	0	1	-17.298	4	0	1	-.015	4	0	1
213		12	max	984.1	1	0	1	.153	9	0	1	0	9	0	1
214			min	-272.719	3	0	1	-17.354	4	0	1	-.017	4	0	1
215		13	max	984.165	1	0	1	.153	9	0	1	0	9	0	1
216			min	-272.671	3	0	1	-17.41	4	0	1	-.018	4	0	1
217		14	max	984.23	1	0	1	.153	9	0	1	0	9	0	1
218			min	-272.622	3	0	1	-17.466	4	0	1	-.02	4	0	1
219		15	max	984.294	1	0	1	.153	9	0	1	0	9	0	1
220			min	-272.574	3	0	1	-17.522	4	0	1	-.021	4	0	1
221		16	max	984.359	1	0	1	.153	9	0	1	0	9	0	1
222			min	-272.525	3	0	1	-17.578	4	0	1	-.023	4	0	1
223		17	max	984.424	1	0	1	.153	9	0	1	0	9	0	1
224			min	-272.477	3	0	1	-17.634	4	0	1	-.025	4	0	1
225		18	max	984.488	1	0	1	.153	9	0	1	0	9	0	1
226			min	-272.428	3	0	1	-17.69	4	0	1	-.026	4	0	1
227		19	max	984.553	1	0	1	.153	9	0	1	0	9	0	1
228			min	-272.38	3	0	1	-17.746	4	0	1	-.028	4	0	1
229	M10	1	max	243.182	1	.673	4	1.043	5	0	1	0	1	0	1
230			min	-317.7	3	.169	15	-.104	1	-.001	5	0	3	0	1
231		2	max	243.289	1	.632	4	.947	5	0	1	0	4	0	15
232			min	-317.62	3	.16	15	-.104	1	-.001	5	0	3	0	4
233		3	max	243.396	1	.59	4	.85	5	0	1	0	4	0	15
234			min	-317.54	3	.15	15	-.104	1	-.001	5	0	3	0	4
235		4	max	243.502	1	.549	4	.754	5	0	1	0	4	0	15
236			min	-317.46	3	.14	15	-.104	1	-.001	5	0	3	0	4
237		5	max	243.609	1	.508	4	.657	5	0	1	0	4	0	15
238			min	-317.38	3	.131	15	-.104	1	-.001	5	0	3	0	4
239		6	max	243.715	1	.467	4	.561	5	0	1	0	4	0	15
240			min	-317.3	3	.121	15	-.104	1	-.001	5	0	3	0	4
241		7	max	243.822	1	.425	4	.464	5	0	1	0	4	0	15
242			min	-317.22	3	.111	15	-.104	1	-.001	5	0	3	0	4
243		8	max	243.928	1	.384	4	.368	5	0	1	0	5	0	15
244			min	-317.141	3	.102	15	-.104	1	-.001	5	0	3	0	4
245		9	max	244.035	1	.343	4	.272	5	0	1	0	5	0	15
246			min	-317.061	3	.092	15	-.104	1	-.001	5	0	3	0	4
247		10	max	244.141	1	.302	4	.175	5	0	1	0	5	0	15
248			min	-316.981	3	.082	15	-.104	1	-.001	5	0	3	0	4
249		11	max	244.248	1	.26	4	.079	5	0	1	0	5	0	15
250			min	-316.901	3	.072	15	-.104	1	-.001	5	0	3	0	4
251		12	max	244.354	1	.219	4	-.003	10	0	1	0	5	0	15
252			min	-316.821	3	.063	15	-.104	1	-.001	5	0	3	0	4
253		13	max	244.461	1	.178	4	-.003	10	0	1	0	5	0	15
254			min	-316.741	3	.053	15	-.126	4	-.001	5	0	3	0	4
255		14	max	244.568	1	.136	4	-.003	10	0	1	0	5	0	15
256			min	-316.661	3	.043	15	-.222	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	244.674	1	.095	4	-.003	10	0	1	0	5	0	15
258		min	-316.581	3	.032	12	-.319	4	-.001	5	0	3	0	4
259	16	max	244.781	1	.054	4	-.003	10	0	1	0	5	0	15
260		min	-316.501	3	.009	9	-.415	4	-.001	5	0	3	0	4
261	17	max	244.887	1	.021	5	-.003	10	0	1	0	5	0	15
262		min	-316.421	3	-.017	9	-.512	4	-.001	5	0	3	0	4
263	18	max	244.994	1	.006	5	-.003	10	0	1	0	5	0	15
264		min	-316.341	3	-.044	9	-.608	4	-.001	5	0	3	0	4
265	19	max	245.1	1	-.005	15	-.003	10	0	1	0	5	0	15
266		min	-316.262	3	-.075	1	-.705	4	-.001	5	0	3	0	4
267	M11	1	max	95.468	2	1.792	.186	1	0	4	0	5	0	6
268		min	-87.546	3	.418	15	-1.246	5	0	10	0	1	0	15
269	2	max	95.4	2	1.614	6	.186	1	0	4	0	5	0	2
270		min	-87.596	3	.377	15	-1.112	5	0	10	0	1	0	15
271	3	max	95.332	2	1.437	6	.186	1	0	4	0	5	0	2
272		min	-87.647	3	.335	15	-.979	5	0	10	0	1	0	3
273	4	max	95.264	2	1.259	6	.186	1	0	4	0	3	0	15
274		min	-87.698	3	.293	15	-.845	5	0	10	0	1	0	4
275	5	max	95.197	2	1.081	6	.186	1	0	4	0	3	0	15
276		min	-87.749	3	.251	15	-.711	5	0	10	0	1	0	4
277	6	max	95.129	2	.904	6	.186	1	0	4	0	3	0	15
278		min	-87.8	3	.21	15	-.578	5	0	10	0	1	0	4
279	7	max	95.061	2	.726	6	.186	1	0	4	0	3	0	15
280		min	-87.851	3	.168	15	-.444	5	0	10	0	1	0	4
281	8	max	94.993	2	.548	6	.186	1	0	4	0	3	0	15
282		min	-87.902	3	.126	15	-.311	5	0	10	0	4	-.001	4
283	9	max	94.925	2	.371	6	.186	1	0	4	0	3	0	15
284		min	-87.953	3	.084	15	-.177	5	0	10	0	4	-.001	4
285	10	max	94.857	2	.193	6	.186	1	0	4	0	3	0	15
286		min	-88.004	3	.043	15	-.043	5	0	10	0	4	-.001	4
287	11	max	94.789	2	.036	2	.186	1	0	4	0	3	0	15
288		min	-88.055	3	-.018	3	-.029	3	0	10	0	4	-.001	4
289	12	max	94.722	2	-.041	15	.264	4	0	4	0	3	0	15
290		min	-88.105	3	-.163	4	-.029	3	0	10	0	4	-.001	4
291	13	max	94.654	2	-.083	15	.398	4	0	4	0	3	0	15
292		min	-88.156	3	-.34	4	-.029	3	0	10	0	4	-.001	4
293	14	max	94.586	2	-.124	15	.532	4	0	4	0	3	0	15
294		min	-88.207	3	-.518	4	-.029	3	0	10	0	5	-.001	4
295	15	max	94.518	2	-.166	15	.665	4	0	4	0	3	0	15
296		min	-88.258	3	-.695	4	-.029	3	0	10	0	5	0	4
297	16	max	94.45	2	-.208	15	.799	4	0	4	0	3	0	15
298		min	-88.309	3	-.873	4	-.029	3	0	10	0	10	0	4
299	17	max	94.382	2	-.25	15	.932	4	0	4	0	4	0	15
300		min	-88.36	3	-1.051	4	-.029	3	0	10	0	10	0	4
301	18	max	94.314	2	-.291	15	1.066	4	0	4	0	4	0	15
302		min	-88.411	3	-1.228	4	-.029	3	0	10	0	10	0	4
303	19	max	94.246	2	-.333	15	1.2	4	0	4	0	4	0	1
304		min	-88.462	3	-1.406	4	-.029	3	0	10	0	10	0	1
305	M12	1	max	334.793	1	0	.805	1	0	1	0	4	0	1
306		min	-75.96	3	0	1	-15.363	5	0	1	0	3	0	1
307	2	max	334.858	1	0	1	.805	1	0	1	0	1	0	1
308		min	-75.912	3	0	1	-15.419	5	0	1	-.001	5	0	1
309	3	max	334.922	1	0	1	.805	1	0	1	0	1	0	1
310		min	-75.863	3	0	1	-15.475	5	0	1	-.003	5	0	1
311	4	max	334.987	1	0	1	.805	1	0	1	0	1	0	1
312		min	-75.815	3	0	1	-15.531	5	0	1	-.004	5	0	1
313	5	max	335.052	1	0	1	.805	1	0	1	0	1	0	1



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Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-75.766	3	0	1	-15.587	5	0	1	-.006	5	0	1
315	6	max	335.117	1	0	1	.805	1	0	1	0	1	0	1
316		min	-75.717	3	0	1	-15.643	5	0	1	-.007	5	0	1
317	7	max	335.181	1	0	1	.805	1	0	1	0	1	0	1
318		min	-75.669	3	0	1	-15.7	5	0	1	-.008	5	0	1
319	8	max	335.246	1	0	1	.805	1	0	1	0	1	0	1
320		min	-75.62	3	0	1	-15.756	5	0	1	-.01	5	0	1
321	9	max	335.311	1	0	1	.805	1	0	1	0	1	0	1
322		min	-75.572	3	0	1	-15.812	5	0	1	-.011	5	0	1
323	10	max	335.375	1	0	1	.805	1	0	1	0	1	0	1
324		min	-75.523	3	0	1	-15.868	5	0	1	-.013	5	0	1
325	11	max	335.44	1	0	1	.805	1	0	1	0	1	0	1
326		min	-75.475	3	0	1	-15.924	5	0	1	-.014	5	0	1
327	12	max	335.505	1	0	1	.805	1	0	1	0	1	0	1
328		min	-75.426	3	0	1	-15.98	5	0	1	-.015	5	0	1
329	13	max	335.569	1	0	1	.805	1	0	1	0	1	0	1
330		min	-75.378	3	0	1	-16.036	5	0	1	-.017	5	0	1
331	14	max	335.634	1	0	1	.805	1	0	1	0	1	0	1
332		min	-75.329	3	0	1	-16.092	5	0	1	-.018	5	0	1
333	15	max	335.699	1	0	1	.805	1	0	1	.001	1	0	1
334		min	-75.281	3	0	1	-16.148	5	0	1	-.02	5	0	1
335	16	max	335.764	1	0	1	.805	1	0	1	.001	1	0	1
336		min	-75.232	3	0	1	-16.204	5	0	1	-.021	5	0	1
337	17	max	335.828	1	0	1	.805	1	0	1	.001	1	0	1
338		min	-75.184	3	0	1	-16.26	5	0	1	-.023	5	0	1
339	18	max	335.893	1	0	1	.805	1	0	1	.001	1	0	1
340		min	-75.135	3	0	1	-16.316	5	0	1	-.024	5	0	1
341	19	max	335.958	1	0	1	.805	1	0	1	.001	1	0	1
342		min	-75.087	3	0	1	-16.373	5	0	1	-.026	5	0	1
343	M1	1	max	67.975	1	340.195	3	.001	10	0	.035	1	0	2
344		min	4.575	10	-245.082	1	-17.834	1	0	3	0	10	0	3
345	2	max	68.07	1	339.998	3	.001	10	0	1	.031	1	.053	1
346		min	4.655	10	-245.344	1	-17.834	1	0	3	0	10	-.074	3
347	3	max	55.132	1	4.69	14	.004	10	0	5	.027	1	.106	1
348		min	-1.127	10	-19.514	3	-17.731	1	0	1	0	10	-.146	3
349	4	max	55.228	1	4.432	14	.004	10	0	5	.023	1	.107	1
350		min	-1.047	10	-19.711	3	-17.731	1	0	1	0	10	-.142	3
351	5	max	55.323	1	4.174	14	.004	10	0	5	.019	1	.109	2
352		min	-.968	10	-19.908	3	-17.731	1	0	1	0	10	-.138	3
353	6	max	55.419	1	3.917	14	.004	10	0	5	.015	1	.113	2
354		min	-.888	10	-20.105	3	-17.731	1	0	1	0	10	-.133	3
355	7	max	55.514	1	3.659	14	.004	10	0	5	.012	1	.117	2
356		min	-.809	10	-20.302	3	-17.731	1	0	1	0	10	-.129	3
357	8	max	55.61	1	3.401	14	.004	10	0	5	.008	1	.121	2
358		min	-.729	10	-20.498	3	-17.731	1	0	1	0	10	-.125	3
359	9	max	55.705	1	3.143	14	.004	10	0	5	.004	1	.125	2
360		min	-.649	10	-20.695	3	-17.731	1	0	1	0	10	-.12	3
361	10	max	55.801	1	2.885	14	.004	10	0	5	.001	3	.129	2
362		min	-.57	10	-20.892	3	-17.731	1	0	1	0	10	-.116	3
363	11	max	55.896	1	2.634	9	.004	10	0	5	0	3	.133	2
364		min	-.49	10	-21.089	3	-17.731	1	0	1	-.004	1	-.111	3
365	12	max	55.992	1	2.415	9	.004	10	0	5	0	10	.137	2
366		min	-.411	10	-21.286	3	-17.731	1	0	1	-.008	1	-.107	3
367	13	max	56.087	1	2.196	9	.004	10	0	5	0	10	.141	2
368		min	-.331	10	-21.482	3	-17.731	1	0	1	-.011	1	-.102	3
369	14	max	56.183	1	1.978	9	.004	10	0	5	0	10	.146	2
370		min	-.252	10	-21.679	3	-17.731	1	0	1	-.015	1	-.097	3



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Dec 11, 2015

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	56.278	1	1.759	9	.004	10	0	5	0	10	.15	2
372			min	-.172	10	-21.876	3	-17.731	1	0	1	-.019	1	-.092	3
373		16	max	80.159	2	47.215	2	.004	10	0	1	0	10	.154	2
374			min	-31.129	3	-85.85	3	-17.889	1	0	5	-.023	1	-.087	3
375		17	max	80.254	2	46.952	2	.004	10	0	1	0	10	.144	2
376			min	-31.057	3	-86.047	3	-17.889	1	0	5	-.027	1	-.069	3
377		18	max	-3.195	12	333.098	2	.012	10	0	3	0	10	.073	2
378			min	-68.041	1	-158.099	3	-26.648	4	0	2	-.031	1	-.035	3
379		19	max	-3.147	12	332.835	2	.012	10	0	3	0	10	0	2
380			min	-67.946	1	-158.295	3	-26.406	4	0	2	-.035	1	0	3
381	M5	1	max	164.56	1	1097.866	3	0	10	0	9	.031	4	0	3
382			min	-.201	3	-787.399	1	-57.406	3	0	5	0	10	0	2
383		2	max	164.655	1	1097.669	3	0	10	0	9	.026	4	.17	1
384			min	-.13	3	-787.662	1	-57.406	3	0	5	-.004	3	-.238	3
385		3	max	122.611	1	5.918	9	6.241	3	0	3	.022	4	.338	1
386			min	-.715	10	-69.794	3	-17.042	4	0	4	-.016	3	-.47	3
387		4	max	122.707	1	5.699	9	6.241	3	0	3	.018	4	.343	1
388			min	-.635	10	-69.991	3	-16.8	4	0	4	-.015	3	-.455	3
389		5	max	122.802	1	5.481	9	6.241	3	0	3	.015	4	.349	2
390			min	-.556	10	-70.188	3	-16.558	4	0	4	-.013	3	-.44	3
391		6	max	122.898	1	5.262	9	6.241	3	0	3	.011	4	.363	2
392			min	-.476	10	-70.385	3	-16.316	4	0	4	-.012	3	-.425	3
393		7	max	122.993	1	5.043	9	6.241	3	0	3	.008	4	.376	2
394			min	-.397	10	-70.582	3	-16.074	4	0	4	-.011	3	-.41	3
395		8	max	123.089	1	4.825	9	6.241	3	0	3	.004	4	.39	2
396			min	-.317	10	-70.778	3	-15.832	4	0	4	-.009	3	-.394	3
397		9	max	123.184	1	4.606	9	6.241	3	0	3	0	4	.403	2
398			min	-.237	10	-70.975	3	-15.59	4	0	4	-.008	3	-.379	3
399		10	max	123.28	1	4.387	9	6.241	3	0	3	0	11	.417	2
400			min	-.158	10	-71.172	3	-15.348	4	0	4	-.007	3	-.363	3
401		11	max	123.375	1	4.169	9	6.241	3	0	3	0	2	.43	2
402			min	-.078	10	-71.369	3	-15.106	4	0	4	-.006	4	-.348	3
403		12	max	123.471	1	3.95	9	6.241	3	0	3	0	2	.444	2
404			min	.001	10	-71.566	3	-14.864	4	0	4	-.009	4	-.332	3
405		13	max	123.566	1	3.731	9	6.241	3	0	3	0	2	.458	2
406			min	.081	10	-71.762	3	-14.622	4	0	4	-.012	4	-.317	3
407		14	max	123.662	1	3.513	9	6.241	3	0	3	0	2	.471	2
408			min	.161	10	-71.959	3	-14.38	4	0	4	-.016	4	-.301	3
409		15	max	123.757	1	3.294	9	6.241	3	0	3	0	3	.485	2
410			min	.24	10	-72.156	3	-14.138	4	0	4	-.019	4	-.286	3
411		16	max	259.586	2	174.27	2	6.21	3	0	3	0	3	.497	2
412			min	-97.38	3	-242.95	3	-12.88	4	0	4	-.022	4	-.269	3
413		17	max	259.682	2	174.008	2	6.21	3	0	3	.002	3	.459	2
414			min	-97.309	3	-243.146	3	-12.638	4	0	4	-.025	4	-.216	3
415		18	max	-2.846	12	1068.549	2	5.73	3	0	4	.004	3	.231	2
416			min	-164.72	1	-500.912	3	-28.777	5	0	9	-.031	4	-.108	3
417		19	max	-2.798	12	1068.287	2	5.73	3	0	4	.005	3	0	3
418			min	-164.625	1	-501.109	3	-28.535	5	0	9	-.037	4	0	2
419	M9	1	max	67.86	1	340.139	3	119.666	4	0	3	0	5	0	2
420			min	.24	15	-245.081	1	-.001	10	0	1	-.035	1	0	3
421		2	max	67.956	1	339.942	3	119.908	4	0	3	.025	5	.053	1
422			min	.269	15	-245.344	1	-.001	10	0	1	-.031	1	-.074	3
423		3	max	55.377	1	4.368	9	17.469	1	0	1	.049	5	.106	1
424			min	-.798	10	-19.426	3	-22.234	5	0	10	-.027	1	-.146	3
425		4	max	55.473	1	4.15	9	17.469	1	0	1	.044	5	.107	1
426			min	-.719	10	-19.623	3	-21.992	5	0	10	-.023	1	-.142	3
427		5	max	55.568	1	3.931	9	17.469	1	0	1	.039	5	.109	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-639	10	-19.82	3	-21.75	5	0	10	-.019	1	-.138	3
429	6	max	55.664	1	3.712	9	17.469	1	0	1	.035	5	.113	2
430		min	-.559	10	-20.016	3	-21.508	5	0	10	-.015	1	-.133	3
431	7	max	55.759	1	3.494	9	17.469	1	0	1	.03	5	.117	2
432		min	-.48	10	-20.213	3	-21.266	5	0	10	-.011	1	-.129	3
433	8	max	55.855	1	3.275	9	17.469	1	0	1	.025	5	.121	2
434		min	-.4	10	-20.41	3	-21.024	5	0	10	-.008	1	-.125	3
435	9	max	55.95	1	3.056	9	17.469	1	0	1	.021	5	.125	2
436		min	-.321	10	-20.607	3	-20.782	5	0	10	-.004	1	-.12	3
437	10	max	56.046	1	2.838	9	17.469	1	0	1	.016	4	.129	2
438		min	-.241	10	-20.804	3	-20.54	5	0	10	0	1	-.116	3
439	11	max	56.141	1	2.619	9	17.469	1	0	1	.013	4	.133	2
440		min	-.161	10	-.21	3	-20.298	5	0	10	0	10	-.111	3
441	12	max	56.237	1	2.4	9	17.469	1	0	1	.009	4	.137	2
442		min	-.082	10	-21.197	3	-20.056	5	0	10	0	10	-.107	3
443	13	max	56.332	1	2.182	9	17.469	1	0	1	.011	1	.141	2
444		min	-.002	10	-21.394	3	-19.814	5	0	10	0	10	-.102	3
445	14	max	56.428	1	1.963	9	17.469	1	0	1	.015	1	.146	2
446		min	.077	10	-21.591	3	-19.572	5	0	10	0	5	-.097	3
447	15	max	56.523	1	1.744	9	17.469	1	0	1	.019	1	.15	2
448		min	.157	10	-21.788	3	-19.33	5	0	10	-.005	5	-.093	3
449	16	max	80.261	2	46.934	2	17.64	1	0	10	.023	1	.154	2
450		min	-31.863	3	-86.249	3	-17.931	5	0	4	-.008	5	-.087	3
451	17	max	80.357	2	46.671	2	17.64	1	0	10	.027	1	.144	2
452		min	-31.792	3	-86.446	3	-17.689	5	0	4	-.012	5	-.069	3
453	18	max	7.846	5	333.098	2	18.438	1	0	2	.031	1	.073	2
454		min	-67.92	1	-158.092	3	-32.799	5	0	3	-.019	5	-.035	3
455	19	max	7.891	5	332.836	2	18.438	1	0	2	.035	1	0	2
456		min	-67.824	1	-158.289	3	-32.557	5	0	3	-.026	5	0	3
457	M13	1	max	119.666	4	244.889	1	-.24	15	0	.035	1	0	1
458		min	-.001	10	-340.17	3	-67.856	1	0	3	0	5	0	3
459	2	max	115.028	4	174.027	1	.362	5	0	2	.01	3	.137	3
460		min	-.001	10	-241.384	3	-51.168	1	0	3	-.002	10	-.099	1
461	3	max	110.39	4	103.165	1	1.216	5	0	2	.008	3	.228	3
462		min	-.001	10	-142.598	3	-34.479	1	0	3	-.014	1	-.164	1
463	4	max	105.752	4	32.303	1	2.07	5	0	2	.005	3	.272	3
464		min	-.001	10	-43.812	3	-17.79	1	0	3	-.026	1	-.196	1
465	5	max	101.114	4	54.974	3	2.925	5	0	2	.003	3	.269	3
466		min	-.001	10	-38.559	1	-3.808	3	0	3	-.03	1	-.195	1
467	6	max	96.476	4	153.76	3	15.587	1	0	2	.004	5	.22	3
468		min	-.001	10	-109.421	1	-3.005	3	0	3	-.027	1	-.16	1
469	7	max	91.838	4	252.546	3	32.276	1	0	2	.006	5	.124	3
470		min	-.001	10	-180.283	1	-2.201	3	0	3	-.016	1	-.092	1
471	8	max	87.2	4	351.332	3	48.964	1	0	2	.008	4	.01	1
472		min	-.001	10	-251.145	1	-1.398	3	0	3	0	3	-.018	3
473	9	max	82.562	4	450.118	3	65.653	1	0	2	.031	1	.146	1
474		min	-.001	10	-322.007	1	-.594	3	0	3	0	3	-.208	3
475	10	max	77.924	4	342.554	12	82.341	1	0	2	.066	1	.314	1
476		min	-.001	10	-548.904	3	.293	12	0	3	-.016	5	-.444	3
477	11	max	53.656	4	322.007	1	6.08	5	0	3	.03	1	.146	1
478		min	-.001	10	-450.118	3	-65.538	1	0	2	-.014	5	-.208	3
479	12	max	49.018	4	251.145	1	6.934	5	0	3	.004	2	.01	1
480		min	-.001	10	-351.332	3	-48.85	1	0	2	-.011	5	-.018	3
481	13	max	44.38	4	180.283	1	7.788	5	0	3	0	10	.124	3
482		min	-.001	10	-252.546	3	-32.161	1	0	2	-.016	1	-.092	1
483	14	max	39.742	4	109.421	1	8.643	5	0	3	-.002	10	.22	3
484		min	-.001	10	-153.76	3	-15.472	1	0	2	-.027	1	-.16	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	35.104	4	38.559	1	10.102	4	0	3	0	5	.269	3
486			min	-.001	10	-54.974	3	-1.829	2	0	2	-.03	1	-.195	1
487		16	max	30.466	4	43.812	3	17.905	1	0	3	.006	5	.272	3
488			min	-.001	10	-32.303	1	0	10	0	2	-.026	1	-.196	1
489		17	max	25.828	4	142.598	3	34.594	1	0	3	.011	5	.228	3
490			min	-.001	10	-103.165	1	1.525	10	0	2	-.013	1	-.164	1
491		18	max	21.19	4	241.384	3	51.282	1	0	3	.018	4	.137	3
492			min	-.001	10	-174.027	1	3.05	10	0	2	-.002	10	-.099	1
493		19	max	17.863	1	340.17	3	67.971	1	0	3	.035	1	0	1
494			min	-.001	10	-244.889	1	4.575	10	0	2	0	10	0	3
495	M16	1	max	32.546	5	332.925	2	7.891	5	0	3	.035	1	0	2
496			min	-18.408	1	-158.306	3	-67.828	1	0	2	-.026	5	0	3
497		2	max	27.908	5	236.535	2	8.745	5	0	3	.007	1	.064	3
498			min	-18.408	1	-112.875	3	-51.14	1	0	2	-.022	5	-.134	2
499		3	max	23.27	5	140.146	2	9.599	5	0	3	0	12	.107	3
500			min	-18.408	1	-67.444	3	-34.451	1	0	2	-.021	4	-.223	2
501		4	max	18.632	5	43.757	2	10.453	5	0	3	-.001	12	.128	3
502			min	-18.408	1	-22.013	3	-17.763	1	0	2	-.026	1	-.267	2
503		5	max	13.994	5	23.418	3	11.307	5	0	3	-.002	12	.127	3
504			min	-18.408	1	-52.633	2	-2.478	3	0	2	-.03	1	-.265	2
505		6	max	9.356	5	68.849	3	15.615	1	0	3	-.002	15	.106	3
506			min	-18.408	1	-149.022	2	-1.675	3	0	2	-.027	1	-.217	2
507		7	max	4.718	5	114.279	3	32.303	1	0	3	.003	5	.062	3
508			min	-18.408	1	-245.411	2	-.871	3	0	2	-.016	1	-.124	2
509		8	max	2.006	3	159.71	3	48.992	1	0	3	.01	4	.015	2
510			min	-18.408	1	-341.801	2	-.068	3	0	2	-.005	3	-.002	3
511		9	max	2.006	3	205.141	3	65.681	1	0	3	.031	1	.199	2
512			min	-18.408	1	-438.19	2	.588	12	0	2	-.005	3	-.088	3
513		10	max	19.346	5	-8.4	15	82.369	1	0	14	.066	1	.429	2
514			min	-18.408	1	-534.579	2	-2.422	3	0	2	-.004	3	-.196	3
515		11	max	14.708	5	438.19	2	5.276	5	0	2	.031	1	.199	2
516			min	-18.368	1	-205.141	3	-65.559	1	0	3	-.011	5	-.088	3
517		12	max	10.07	5	341.801	2	6.13	5	0	2	.004	2	.015	2
518			min	-18.368	1	-159.71	3	-48.87	1	0	3	-.008	5	-.002	3
519		13	max	5.432	5	245.411	2	6.984	5	0	2	0	10	.062	3
520			min	-18.368	1	-114.279	3	-32.182	1	0	3	-.016	1	-.124	2
521		14	max	.794	5	149.022	2	7.839	5	0	2	0	12	.106	3
522			min	-18.368	1	-68.848	3	-15.493	1	0	3	-.027	1	-.217	2
523		15	max	.012	10	52.633	2	9.276	4	0	2	.002	5	.127	3
524			min	-18.368	1	-23.418	3	-1.838	2	0	3	-.03	1	-.265	2
525		16	max	.012	10	22.013	3	17.884	1	0	2	.007	5	.128	3
526			min	-18.368	1	-43.757	2	-.007	10	0	3	-.026	1	-.267	2
527		17	max	.012	10	67.444	3	34.573	1	0	2	.011	5	.107	3
528			min	-18.368	1	-140.146	2	1.519	10	0	3	-.013	1	-.223	2
529		18	max	.012	10	112.875	3	51.262	1	0	2	.018	4	.064	3
530			min	-21.79	4	-236.535	2	2.611	12	0	3	-.002	10	-.134	2
531		19	max	.012	10	158.306	3	67.95	1	0	2	.035	1	0	2
532			min	-26.428	4	-332.925	2	3.147	12	0	3	0	10	0	3
533	M15	1	max	0	1	.876	3	.121	3	0	1	0	1	0	1
534			min	-75.906	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.778	3	.121	3	0	1	0	1	0	1
536			min	-75.965	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.681	3	.121	3	0	1	0	1	0	1
538			min	-76.025	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.584	3	.121	3	0	1	0	1	0	1
540			min	-76.085	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.487	3	.121	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-76.144	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.389	3	.121	3	0	1	0	1	0	1
544			min	-76.204	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.292	3	.121	3	0	1	0	3	0	1
546			min	-76.264	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.195	3	.121	3	0	1	0	3	0	1
548			min	-76.323	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.097	3	.121	3	0	1	0	3	0	1
550			min	-76.383	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.121	3	0	1	0	3	0	1
552			min	-76.443	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.121	3	0	1	0	3	0	1
554			min	-76.502	3	-.097	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.121	3	0	1	0	3	0	1
556			min	-76.562	3	-.195	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.121	3	0	1	0	3	0	1
558			min	-76.622	3	-.292	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.121	3	0	1	0	3	0	1
560			min	-76.681	3	-.389	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.121	3	0	1	0	3	0	1
562			min	-76.741	3	-.487	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.121	3	0	1	0	3	0	1
564			min	-76.801	3	-.584	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.121	3	0	1	0	3	0	1
566			min	-76.86	3	-.681	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.121	3	0	1	0	3	0	1
568			min	-76.92	3	-.778	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.121	3	0	1	0	3	0	1
570			min	-76.98	3	-.876	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.108	4	.256	4	0	3	0	3	0	1
572			min	-168.153	4	0	2	-.048	3	0	1	0	4	0	1
573		2	max	0	2	1.874	4	.232	4	0	3	0	3	0	2
574			min	-168.162	4	0	2	-.048	3	0	1	0	4	0	4
575		3	max	0	2	1.64	4	.207	4	0	3	0	3	0	2
576			min	-168.17	4	0	2	-.048	3	0	1	0	4	-.001	4
577		4	max	0	2	1.406	4	.183	4	0	3	0	3	0	2
578			min	-168.179	4	0	2	-.048	3	0	1	0	1	-.001	4
579		5	max	0	2	1.171	4	.159	4	0	3	0	3	0	2
580			min	-168.187	4	0	2	-.048	3	0	1	0	1	-.002	4
581		6	max	0	2	.937	4	.134	4	0	3	0	3	0	2
582			min	-168.195	4	0	2	-.048	3	0	1	0	1	-.002	4
583		7	max	0	2	.703	4	.11	4	0	3	0	3	0	2
584			min	-168.204	4	0	2	-.048	3	0	1	0	1	-.002	4
585		8	max	0	2	.469	4	.085	4	0	3	0	5	0	2
586			min	-168.212	4	0	2	-.048	3	0	1	0	1	-.003	4
587		9	max	0	2	.234	4	.061	4	0	3	0	5	0	2
588			min	-168.221	4	0	2	-.048	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.041	1	0	3	0	5	0	2
590			min	-168.229	4	0	1	-.048	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.041	1	0	3	0	5	0	2
592			min	-168.237	4	-.234	4	-.048	3	0	1	0	1	-.003	4
593		12	max	.026	11	0	2	.041	1	0	3	0	5	0	2
594			min	-168.246	4	-.469	4	-.048	3	0	1	0	1	-.003	4
595		13	max	.092	11	0	2	.041	1	0	3	0	5	0	2
596			min	-168.254	4	-.703	4	-.048	3	0	1	0	3	-.002	4
597		14	max	.159	11	0	2	.041	1	0	3	0	5	0	2
598			min	-168.263	4	-.937	4	-.065	5	0	1	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.225	11	0	2	.041	1	0	3	0	4	0	2
600		min	-168.271	4	-1.171	4	-.09	5	0	1	0	3	-.002	4
601	16	max	.291	11	0	2	.041	1	0	3	0	4	0	2
602		min	-168.279	4	-1.406	4	-.114	5	0	1	0	3	-.001	4
603	17	max	.358	11	0	2	.041	1	0	3	0	1	0	2
604		min	-168.288	4	-1.64	4	-.138	5	0	1	0	3	-.001	4
605	18	max	.424	11	0	2	.041	1	0	3	0	1	0	2
606		min	-168.296	4	-1.874	4	-.163	5	0	1	0	3	0	4
607	19	max	.49	11	0	2	.041	1	0	3	0	1	0	1
608		min	-168.325	5	-2.108	4	-.187	5	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.007	2	.003	1	9.442e-4	5	NC	3	NC	1
2			min	-.003	3	-.006	3	-.009	5	-2.659e-4	1	4977.116	2	NC	1
3		2	max	.002	1	.006	2	.003	1	9.639e-4	5	NC	3	NC	1
4			min	-.003	3	-.006	3	-.009	5	-2.546e-4	1	5414.966	2	NC	1
5		3	max	.002	1	.006	2	.003	1	9.837e-4	5	NC	1	NC	1
6			min	-.003	3	-.006	3	-.009	5	-2.432e-4	1	5932.869	2	NC	1
7		4	max	.002	1	.005	2	.002	1	1.004e-3	5	NC	1	NC	1
8			min	-.002	3	-.005	3	-.008	5	-2.319e-4	1	6549.85	2	NC	1
9		5	max	.002	1	.005	2	.002	1	1.023e-3	5	NC	1	NC	1
10			min	-.002	3	-.005	3	-.008	5	-2.205e-4	1	7291.19	2	NC	1
11		6	max	.001	1	.004	2	.002	1	1.043e-3	5	NC	1	NC	1
12			min	-.002	3	-.005	3	-.008	5	-2.092e-4	1	8191.037	2	NC	1
13		7	max	.001	1	.004	2	.002	1	1.063e-3	5	NC	1	NC	1
14			min	-.002	3	-.005	3	-.007	5	-1.978e-4	1	9296.416	2	NC	1
15		8	max	.001	1	.003	2	.001	1	1.083e-3	5	NC	1	NC	1
16			min	-.002	3	-.004	3	-.007	5	-1.865e-4	1	NC	1	NC	1
17		9	max	.001	1	.003	2	.001	1	1.102e-3	5	NC	1	NC	1
18			min	-.002	3	-.004	3	-.006	5	-1.751e-4	1	NC	1	NC	1
19		10	max	0	1	.002	2	.001	1	1.122e-3	5	NC	1	NC	1
20		min	-.001	3	-.004	3	-.006	5	-1.638e-4	1	NC	1	NC	1	
21	11	max	0	1	.002	2	0	1	1.142e-3	5	NC	1	NC	1	
22		min	-.001	3	-.003	3	-.005	5	-1.524e-4	1	NC	1	NC	1	
23	12	max	0	1	.002	2	0	1	1.162e-3	5	NC	1	NC	1	
24		min	-.001	3	-.003	3	-.005	5	-1.411e-4	1	NC	1	NC	1	
25	13	max	0	1	.001	2	0	1	1.182e-3	5	NC	1	NC	1	
26		min	0	3	-.003	3	-.004	5	-1.297e-4	1	NC	1	NC	1	
27	14	max	0	1	0	2	0	1	1.201e-3	5	NC	1	NC	1	
28		min	0	3	-.002	3	-.003	5	-1.184e-4	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	1	1.221e-3	5	NC	1	NC	1	
30		min	0	3	-.002	3	-.003	5	-1.07e-4	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.241e-3	5	NC	1	NC	1	
32		min	0	3	-.001	3	-.002	5	-9.566e-5	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.261e-3	5	NC	1	NC	1	
34		min	0	3	0	3	-.001	5	-8.431e-5	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.28e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-7.296e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.3e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-6.161e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.836e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.977e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	3.763e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-6.014e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	4.689e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-6.051e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.009	5	5.615e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.089e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.013	4	6.542e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.126e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.016	4	7.468e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-6.163e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.019	4	8.395e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-6.2e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.022	4	9.321e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-6.238e-4	5	NC	1	NC	1
55		9	max	0	3	0	2	.025	4	1.025e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-6.275e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.028	4	1.117e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-6.312e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.03	4	1.21e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-6.349e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.033	4	1.303e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-6.387e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.036	4	1.395e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-6.424e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.038	4	1.488e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-6.461e-4	5	NC	1	NC	1
67		15	max	0	3	.004	2	.041	4	1.581e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-6.498e-4	5	NC	1	NC	1
69		16	max	0	3	.005	2	.043	4	1.673e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-6.536e-4	5	8679.861	2	NC	1
71		17	max	0	3	.006	2	.046	4	1.766e-4	1	NC	1	NC	1
72			min	0	2	-.007	3	0	10	-6.573e-4	5	7407.377	2	NC	1
73		18	max	0	3	.007	2	.048	4	1.859e-4	1	NC	3	NC	1
74			min	-.001	2	-.007	3	0	10	-6.61e-4	5	6424.107	2	NC	1
75		19	max	0	3	.008	2	.05	4	1.951e-4	1	NC	3	NC	1
76			min	-.001	2	-.007	3	0	10	-6.647e-4	5	5656.349	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	2.54e-3	5	NC	1	NC	1
78			min	0	3	-.006	3	-.053	4	-2.214e-4	1	NC	1	364.809	4
79		2	max	.002	1	.007	2	0	10	2.54e-3	5	NC	1	NC	1
80			min	0	3	-.006	3	-.049	4	-2.214e-4	1	NC	1	397.652	4
81		3	max	.001	1	.007	2	0	10	2.54e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.044	4	-2.214e-4	1	NC	1	436.737	4
83		4	max	.001	1	.006	2	0	10	2.54e-3	5	NC	1	NC	1
84			min	0	3	-.005	3	-.04	4	-2.214e-4	1	NC	1	483.707	4
85		5	max	.001	1	.006	2	0	10	2.54e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.036	4	-2.214e-4	1	NC	1	540.804	4
87		6	max	.001	1	.005	2	0	10	2.54e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.032	4	-2.214e-4	1	NC	1	611.139	4
89		7	max	.001	1	.005	2	0	10	2.54e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.028	4	-2.214e-4	1	NC	1	699.148	4
91		8	max	0	1	.005	2	0	10	2.54e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.024	4	-2.214e-4	1	NC	1	811.325	4
93		9	max	0	1	.004	2	0	10	2.54e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.02	4	-2.214e-4	1	NC	1	957.49	4
95		10	max	0	1	.004	2	0	10	2.54e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.017	4	-2.214e-4	1	NC	1	1153.08	4
97		11	max	0	1	.003	2	0	10	2.54e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.014	4	-2.214e-4	1	NC	1	1423.518	4
99		12	max	0	1	.003	2	0	10	2.54e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Dec 11, 2015

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.002	3	-.011	4	-2.214e-4	1	NC	1	1813.148	4
101		max	0	1	.003	2	0	10	2.54e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.008	4	-2.214e-4	1	NC	1	2405.143	4
103		max	0	1	.002	2	0	10	2.54e-3	5	NC	1	NC	1
104		min	0	3	-.002	3	-.006	4	-2.214e-4	1	NC	1	3371.054	4
105		max	0	1	.002	2	0	10	2.54e-3	5	NC	1	NC	1
106		min	0	3	-.001	3	-.004	4	-2.214e-4	1	NC	1	5113.891	4
107		max	0	1	.001	2	0	10	2.54e-3	5	NC	1	NC	1
108		min	0	3	-.001	3	-.002	4	-2.214e-4	1	NC	1	8779.266	4
109		max	0	1	0	2	0	10	2.54e-3	5	NC	1	NC	1
110		min	0	3	0	3	-.001	4	-2.214e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.54e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-2.214e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.54e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-2.214e-4	1	NC	1	NC	1
115	M6	max	.006	1	.022	2	0	9	1.005e-3	4	NC	3	NC	1
116		min	-.009	3	-.018	3	-.009	5	-8.387e-8	2	1510.665	2	7315.384	3
117		max	.006	1	.021	2	0	9	1.024e-3	4	NC	3	NC	1
118		min	-.009	3	-.017	3	-.009	5	-2.02e-7	11	1614.447	2	7817.47	3
119		max	.006	1	.019	2	0	9	1.043e-3	4	NC	3	NC	1
120		min	-.008	3	-.016	3	-.009	5	-1.092e-6	11	1733.125	2	8407.736	3
121		max	.005	1	.018	2	0	9	1.063e-3	4	NC	3	NC	1
122		min	-.008	3	-.016	3	-.008	5	-1.981e-6	11	1869.693	2	9105.773	3
123		max	.005	1	.016	2	0	9	1.082e-3	4	NC	3	NC	1
124		min	-.007	3	-.015	3	-.008	5	-3.341e-6	1	2028.01	2	9937.442	3
125		max	.005	1	.015	2	0	9	1.102e-3	4	NC	3	NC	1
126		min	-.007	3	-.014	3	-.008	5	-5.392e-6	1	2213.128	2	NC	1
127		max	.004	1	.014	2	0	9	1.121e-3	4	NC	3	NC	1
128		min	-.006	3	-.013	3	-.007	5	-7.443e-6	1	2431.793	2	NC	1
129		max	.004	1	.012	2	0	9	1.14e-3	4	NC	3	NC	1
130		min	-.006	3	-.012	3	-.007	5	-9.494e-6	1	2693.21	2	NC	1
131		max	.003	1	.011	2	0	9	1.16e-3	4	NC	3	NC	1
132		min	-.005	3	-.011	3	-.006	5	-1.154e-5	1	3010.276	2	NC	1
133		max	.003	1	.01	2	0	9	1.179e-3	4	NC	3	NC	1
134		min	-.005	3	-.01	3	-.006	5	-1.36e-5	1	3401.626	2	NC	1
135		max	.003	1	.009	2	0	1	1.199e-3	4	NC	3	NC	1
136		min	-.004	3	-.009	3	-.005	5	-1.565e-5	1	3895.222	2	NC	1
137		max	.002	1	.007	2	0	1	1.218e-3	4	NC	3	NC	1
138		min	-.004	3	-.008	3	-.005	5	-1.77e-5	1	4535.01	2	NC	1
139		max	.002	1	.006	2	0	1	1.238e-3	4	NC	3	NC	1
140		min	-.003	3	-.007	3	-.004	5	-1.975e-5	1	5394.238	2	NC	1
141		max	.002	1	.005	2	0	1	1.257e-3	4	NC	3	NC	1
142		min	-.003	3	-.006	3	-.004	5	-2.18e-5	1	6604.76	2	NC	1
143		max	.001	1	.004	2	0	1	1.276e-3	4	NC	1	NC	1
144		min	-.002	3	-.004	3	-.003	5	-2.385e-5	1	8430.279	2	NC	1
145		max	.001	1	.003	2	0	1	1.296e-3	4	NC	1	NC	1
146		min	-.002	3	-.003	3	-.002	5	-2.59e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.315e-3	4	NC	1	NC	1
148		min	-.001	3	-.002	3	-.001	5	-2.795e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.335e-3	4	NC	1	NC	1
150		min	0	3	-.001	3	0	5	-3.e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.354e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.205e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.468e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.223e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.003	4	1.346e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-6.142e-4	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.007	4	1.223e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-6.061e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.01	4	1.101e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-5.981e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.013	4	9.782e-6	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-5.9e-4	4	9945.93	2	NC	1
163		6	max	0	3	.006	2	.016	4	1.328e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	1	-5.819e-4	4	7967.823	2	NC	1
165		7	max	0	3	.007	2	.02	4	3.25e-5	3	NC	3	NC	1
166			min	-.001	2	-.009	3	0	1	-5.738e-4	4	6610.319	2	NC	1
167		8	max	.001	3	.008	2	.023	4	5.173e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-5.657e-4	4	5611.906	2	NC	1
169		9	max	.001	3	.01	2	.026	4	7.096e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-5.576e-4	4	4842.404	2	NC	1
171		10	max	.001	3	.011	2	.029	4	9.018e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	1	-5.495e-4	4	4229.668	2	NC	1
173		11	max	.001	3	.012	2	.032	4	1.094e-4	3	NC	3	NC	1
174			min	-.002	2	-.015	3	0	1	-5.414e-4	4	3730.321	2	NC	1
175		12	max	.002	3	.014	2	.034	4	1.286e-4	3	NC	3	NC	1
176			min	-.002	2	-.016	3	0	1	-5.333e-4	4	3316.52	2	NC	1
177		13	max	.002	3	.016	2	.037	4	1.479e-4	3	NC	3	NC	1
178			min	-.002	2	-.017	3	0	1	-5.252e-4	4	2969.388	2	NC	1
179		14	max	.002	3	.017	2	.04	4	1.671e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	0	1	-5.171e-4	4	2675.536	2	NC	1
181		15	max	.002	3	.019	2	.042	4	1.863e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	0	1	-5.09e-4	4	2425.097	2	NC	1
183		16	max	.002	3	.021	2	.045	4	2.055e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	0	1	-5.009e-4	4	2210.581	2	NC	1
185		17	max	.002	3	.023	2	.047	4	2.248e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	0	1	-4.929e-4	4	2026.162	2	NC	1
187		18	max	.003	3	.025	2	.049	4	2.44e-4	3	NC	3	NC	1
188			min	-.004	2	-.021	3	0	1	-4.848e-4	4	1867.224	2	NC	1
189		19	max	.003	3	.027	2	.052	4	2.632e-4	3	NC	3	NC	1
190			min	-.004	2	-.022	3	0	9	-4.767e-4	4	1730.066	2	NC	1
191	M8	1	max	.005	1	.025	2	0	9	2.371e-3	4	NC	1	NC	1
192			min	-.001	3	-.019	3	-.054	4	-2.017e-4	3	NC	1	357.823	4
193		2	max	.004	1	.024	2	0	9	2.371e-3	4	NC	1	NC	1
194			min	-.001	3	-.018	3	-.05	4	-2.017e-4	3	NC	1	390.038	4
195		3	max	.004	1	.022	2	0	9	2.371e-3	4	NC	1	NC	1
196			min	-.001	3	-.017	3	-.045	4	-2.017e-4	3	NC	1	428.376	4
197		4	max	.004	1	.021	2	0	9	2.371e-3	4	NC	1	NC	1
198			min	-.001	3	-.016	3	-.041	4	-2.017e-4	3	NC	1	474.45	4
199		5	max	.004	1	.019	2	0	9	2.371e-3	4	NC	1	NC	1
200			min	-.001	3	-.015	3	-.036	4	-2.017e-4	3	NC	1	530.455	4
201		6	max	.003	1	.018	2	0	9	2.371e-3	4	NC	1	NC	1
202			min	0	3	-.014	3	-.032	4	-2.017e-4	3	NC	1	599.448	4
203		7	max	.003	1	.017	2	0	9	2.371e-3	4	NC	1	NC	1
204			min	0	3	-.013	3	-.028	4	-2.017e-4	3	NC	1	685.777	4
205		8	max	.003	1	.015	2	0	9	2.371e-3	4	NC	1	NC	1
206			min	0	3	-.012	3	-.024	4	-2.017e-4	3	NC	1	795.814	4
207		9	max	.003	1	.014	2	0	9	2.371e-3	4	NC	1	NC	1
208			min	0	3	-.011	3	-.021	4	-2.017e-4	3	NC	1	939.19	4
209		10	max	.002	1	.012	2	0	9	2.371e-3	4	NC	1	NC	1
210			min	0	3	-.01	3	-.017	4	-2.017e-4	3	NC	1	1131.05	4
211		11	max	.002	1	.011	2	0	9	2.371e-3	4	NC	1	NC	1
212			min	0	3	-.009	3	-.014	4	-2.017e-4	3	NC	1	1396.331	4
213		12	max	.002	1	.01	2	0	9	2.371e-3	4	NC	1	NC	1





Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.167e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.654e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.008	4	3.252e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-6.115e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.01	4	1.337e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-6.577e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.013	4	0	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-7.039e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.015	4	0	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-7.5e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.018	5	4.09e-8	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-7.962e-4	4	NC	1	NC	1
283		9	max	0	3	0	2	.021	5	5.873e-8	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	3	-8.423e-4	4	NC	1	NC	1
285		10	max	0	3	.001	2	.023	5	7.656e-8	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-8.885e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.026	5	9.439e-8	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	3	-9.346e-4	4	NC	1	NC	1
289		12	max	0	3	.002	2	.028	5	1.122e-7	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-9.808e-4	4	NC	1	NC	1
291		13	max	0	3	.003	2	.031	5	1.3e-7	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	3	-1.027e-3	4	NC	1	NC	1
293		14	max	0	3	.004	2	.033	5	1.479e-7	10	NC	1	NC	1
294			min	0	2	-.007	3	-.003	3	-1.073e-3	4	NC	1	NC	1
295		15	max	0	3	.004	2	.036	5	1.657e-7	10	NC	1	NC	1
296			min	0	2	-.007	3	-.003	3	-1.119e-3	4	NC	1	NC	1
297		16	max	0	3	.005	2	.038	5	1.835e-7	10	NC	1	NC	1
298			min	0	2	-.007	3	-.002	3	-1.165e-3	4	8691.18	2	NC	1
299		17	max	0	3	.006	2	.04	5	2.014e-7	10	NC	1	NC	1
300			min	0	2	-.007	3	-.003	1	-1.212e-3	4	7416.043	2	NC	1
301		18	max	0	3	.007	2	.043	5	2.192e-7	10	NC	3	NC	1
302			min	-.001	2	-.007	3	-.003	1	-1.258e-3	4	6430.938	2	NC	1
303		19	max	0	3	.008	2	.045	5	2.37e-7	10	NC	3	NC	1
304			min	-.001	2	-.007	3	-.003	1	-1.304e-3	4	5661.887	2	NC	1
305	M12	1	max	.002	1	.008	2	.003	1	3.073e-3	4	NC	1	NC	2
306			min	0	3	-.006	3	-.05	5	-5.123e-7	10	NC	1	389.116	5
307		2	max	.002	1	.007	2	.002	1	3.073e-3	4	NC	1	NC	2
308			min	0	3	-.006	3	-.046	5	-5.123e-7	10	NC	1	424.138	5
309		3	max	.001	1	.007	2	.002	1	3.073e-3	4	NC	1	NC	2
310			min	0	3	-.006	3	-.041	5	-5.123e-7	10	NC	1	465.815	5
311		4	max	.001	1	.006	2	.002	1	3.073e-3	4	NC	1	NC	2
312			min	0	3	-.005	3	-.037	5	-5.123e-7	10	NC	1	515.9	5
313		5	max	.001	1	.006	2	.002	1	3.073e-3	4	NC	1	NC	1
314			min	0	3	-.005	3	-.034	5	-5.123e-7	10	NC	1	576.781	5
315		6	max	.001	1	.005	2	.002	1	3.073e-3	4	NC	1	NC	1
316			min	0	3	-.005	3	-.03	5	-5.123e-7	10	NC	1	651.777	5
317		7	max	.001	1	.005	2	.001	1	3.073e-3	4	NC	1	NC	1
318			min	0	3	-.004	3	-.026	5	-5.123e-7	10	NC	1	745.617	5
319		8	max	0	1	.005	2	.001	1	3.073e-3	4	NC	1	NC	1
320			min	0	3	-.004	3	-.022	5	-5.123e-7	10	NC	1	865.224	5
321		9	max	0	1	.004	2	0	1	3.073e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.019	5	-5.123e-7	10	NC	1	1021.066	5
323		10	max	0	1	.004	2	0	1	3.073e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.016	5	-5.123e-7	10	NC	1	1229.604	5
325		11	max	0	1	.003	2	0	1	3.073e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.013	5	-5.123e-7	10	NC	1	1517.939	5
327		12	max	0	1	.003	2	0	1	3.073e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.01	5	-5.123e-7	10	NC	1	1933.346	5
329		13	max	0	1	.003	2	0	1	3.073e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.008	5	-5.123e-7	10	NC	1	2564.495	5
331		14	max	0	1	.002	2	0	1	3.073e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.005	5	-5.123e-7	10	NC	1	3594.272	5
333		15	max	0	1	.002	2	0	1	3.073e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.004	5	-5.123e-7	10	NC	1	5452.311	5
335		16	max	0	1	.001	2	0	1	3.073e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.002	5	-5.123e-7	10	NC	1	9359.891	5
337		17	max	0	1	0	2	0	1	3.073e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-5.123e-7	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.073e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-5.123e-7	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.073e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-5.123e-7	10	NC	1	NC	1
343	M1	1	max	.006	3	.022	3	.005	5	6.885e-3	1	NC	1	NC	1
344			min	-.007	2	-.019	2	0	9	-9.335e-3	3	NC	1	NC	1
345		2	max	.006	3	.012	3	.007	5	3.342e-3	1	NC	4	NC	1
346			min	-.007	2	-.01	2	-.002	1	-4.593e-3	3	4988.537	3	NC	1
347		3	max	.006	3	.003	3	.009	5	2.295e-4	5	NC	4	NC	1
348			min	-.007	2	-.002	1	-.003	1	-1.35e-4	1	2588.974	3	NC	1
349		4	max	.006	3	.005	2	.012	5	2.25e-4	5	NC	4	NC	1
350			min	-.007	2	-.004	3	-.003	1	-1.104e-4	1	1851.895	3	6817.477	5
351		5	max	.006	3	.011	2	.015	5	2.205e-4	5	NC	4	NC	1
352			min	-.007	2	-.011	3	-.004	1	-8.573e-5	1	1501.828	3	4857.33	5
353		6	max	.006	3	.016	2	.018	5	2.16e-4	5	NC	4	NC	1
354			min	-.007	2	-.015	3	-.003	1	-6.11e-5	1	1281.28	2	3719.327	5
355		7	max	.006	3	.02	2	.021	5	2.116e-4	5	NC	4	NC	1
356			min	-.007	2	-.019	3	-.003	1	-3.659e-5	9	1143.908	2	2986.03	5
357		8	max	.006	3	.023	2	.024	5	2.071e-4	5	NC	4	NC	1
358			min	-.007	2	-.022	3	-.002	1	-1.876e-5	9	1058.157	2	2480.066	5
359		9	max	.006	3	.025	2	.027	5	2.041e-4	4	NC	5	NC	1
360			min	-.007	2	-.023	3	-.002	1	-9.332e-7	9	1007.841	2	2111.904	4
361		10	max	.006	3	.026	2	.031	5	2.053e-4	4	NC	5	NC	1
362			min	-.007	2	-.023	3	0	9	3.311e-7	10	985.272	2	1822.954	4
363		11	max	.006	3	.026	2	.034	4	2.065e-4	4	NC	4	NC	1
364			min	-.007	2	-.022	3	0	9	2.707e-7	10	987.729	2	1603.079	4
365		12	max	.006	3	.024	2	.038	4	2.077e-4	4	NC	4	NC	1
366			min	-.007	2	-.021	3	0	10	2.102e-7	10	1016.433	2	1432.104	4
367		13	max	.006	3	.021	2	.041	4	2.089e-4	4	NC	4	NC	1
368			min	-.007	2	-.018	3	0	10	1.498e-7	10	1077.177	2	1296.937	4
369		14	max	.006	3	.017	2	.044	4	2.101e-4	4	NC	4	NC	1
370			min	-.007	2	-.014	3	0	10	8.932e-8	10	1183.224	2	1188.771	4
371		15	max	.006	3	.011	2	.047	4	2.113e-4	4	NC	4	NC	1
372			min	-.007	2	-.009	3	0	10	0	10	1363.559	2	1101.508	4
373		16	max	.006	3	.005	2	.05	4	3.824e-4	4	NC	4	NC	1
374			min	-.007	2	-.004	3	0	10	0	10	1688.637	2	1030.824	4
375		17	max	.006	3	.002	3	.053	4	4.596e-3	4	NC	4	NC	1
376			min	-.007	2	-.004	2	0	10	5.578e-7	10	2380.116	2	973.657	4
377		18	max	.006	3	.009	3	.055	4	4.631e-3	2	NC	4	NC	1
378			min	-.007	2	-.014	2	0	10	-2.302e-3	3	4603.392	2	927.582	4
379		19	max	.006	3	.017	3	.057	4	9.327e-3	2	NC	1	NC	1
380			min	-.007	2	-.024	2	0	1	-4.693e-3	3	NC	1	891.91	4
381	M5	1	max	.018	3	.069	3	.005	5	1.09e-5	4	NC	1	NC	1
382			min	-.022	2	-.06	2	0	9	0	1	NC	1	NC	1
383		2	max	.018	3	.038	3	.007	5	1.118e-4	5	NC	4	NC	1
384			min	-.022	2	-.033	2	0	9	-1.841e-5	9	1572.11	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.018	3	.01	3	.009	5	2.11e-4	5	NC	5	NC	1
386		min	-.022	2	-.007	1	0	9	-3.653e-5	9	816.234	3	NC	1
387	4	max	.018	3	.016	2	.012	5	2.184e-4	5	NC	5	NC	1
388		min	-.022	2	-.014	3	0	9	-3.459e-5	9	584.445	3	NC	1
389	5	max	.018	3	.036	2	.015	5	2.258e-4	5	NC	5	NC	1
390		min	-.022	2	-.033	3	0	9	-3.265e-5	9	466.936	2	NC	1
391	6	max	.018	3	.053	2	.018	5	2.332e-4	5	NC	5	NC	1
392		min	-.022	2	-.049	3	0	9	-3.07e-5	9	397.444	2	NC	1
393	7	max	.018	3	.066	2	.022	5	2.406e-4	5	NC	5	NC	1
394		min	-.022	2	-.061	3	0	9	-2.876e-5	9	354.686	2	NC	1
395	8	max	.018	3	.076	2	.025	5	2.48e-4	5	NC	5	NC	1
396		min	-.022	2	-.068	3	0	9	-2.682e-5	9	327.982	2	NC	1
397	9	max	.018	3	.082	2	.029	5	2.554e-4	5	NC	5	NC	1
398		min	-.022	2	-.073	3	0	9	-2.488e-5	9	312.294	2	NC	1
399	10	max	.018	3	.085	2	.032	5	2.629e-4	5	NC	5	NC	1
400		min	-.022	2	-.073	3	0	9	-2.294e-5	9	305.23	2	NC	1
401	11	max	.018	3	.083	2	.036	4	2.703e-4	5	NC	5	NC	1
402		min	-.022	2	-.071	3	0	9	-2.1e-5	9	305.939	2	NC	1
403	12	max	.018	3	.078	2	.04	4	2.777e-4	5	NC	5	NC	1
404		min	-.022	2	-.065	3	0	9	-1.906e-5	9	314.796	2	NC	1
405	13	max	.018	3	.069	2	.043	4	2.851e-4	5	NC	5	NC	1
406		min	-.022	2	-.056	3	0	9	-1.712e-5	9	333.595	2	NC	1
407	14	max	.018	3	.055	2	.046	4	2.925e-4	5	NC	5	NC	1
408		min	-.022	2	-.044	3	0	9	-1.518e-5	9	366.45	2	NC	1
409	15	max	.018	3	.037	2	.049	4	2.999e-4	5	NC	5	NC	1
410		min	-.022	2	-.03	3	0	9	-1.324e-5	9	422.354	2	NC	1
411	16	max	.017	3	.015	2	.052	4	4.751e-4	4	NC	5	NC	1
412		min	-.022	2	-.012	3	0	9	-1.235e-5	9	523.186	2	NC	1
413	17	max	.018	3	.007	3	.054	4	4.625e-3	4	NC	5	NC	1
414		min	-.022	2	-.012	2	0	9	-3.649e-5	9	737.949	2	NC	1
415	18	max	.018	3	.029	3	.056	4	2.375e-3	4	NC	4	NC	1
416		min	-.022	2	-.044	2	0	9	-1.866e-5	9	1427.861	2	NC	1
417	19	max	.018	3	.052	3	.057	4	4.33e-6	5	NC	1	NC	1
418		min	-.022	2	-.078	2	0	9	-5.902e-7	3	NC	1	NC	1
419	M9	1	max	.006	.021	3	.004	5	9.343e-3	3	NC	1	NC	1
420		min	-.007	2	-.019	2	0	9	-6.885e-3	1	NC	1	NC	1
421	2	max	.006	3	.012	3	.004	4	4.631e-3	3	NC	4	NC	1
422		min	-.007	2	-.01	2	0	9	-3.381e-3	1	4990.783	3	NC	1
423	3	max	.006	3	.003	3	.004	4	5.884e-5	1	NC	4	NC	1
424		min	-.007	2	-.002	1	0	3	-3.21e-5	5	2590.167	3	NC	1
425	4	max	.006	3	.005	2	.005	4	3.757e-5	1	NC	4	NC	1
426		min	-.007	2	-.005	3	-.001	3	-4.09e-5	5	1852.739	3	NC	1
427	5	max	.006	3	.011	2	.006	4	1.629e-5	1	NC	4	NC	1
428		min	-.007	2	-.011	3	-.002	3	-4.971e-5	5	1502.474	3	NC	1
429	6	max	.006	3	.016	2	.008	4	7.537e-6	2	NC	4	NC	1
430		min	-.007	2	-.016	3	-.003	3	-6.181e-5	4	1281.558	2	NC	1
431	7	max	.006	3	.02	2	.011	4	1.572e-6	2	NC	4	NC	1
432		min	-.007	2	-.019	3	-.003	3	-7.496e-5	4	1144.168	2	7106.989	4
433	8	max	.006	3	.023	2	.014	4	-3.179e-7	10	NC	5	NC	1
434		min	-.007	2	-.022	3	-.004	3	-8.81e-5	4	1058.407	2	4944.501	4
435	9	max	.006	3	.025	2	.017	4	-2.654e-7	10	NC	5	NC	1
436		min	-.007	2	-.023	3	-.004	3	-1.012e-4	4	1008.088	2	3681.227	4
437	10	max	.006	3	.026	2	.02	5	-2.129e-7	10	NC	5	NC	1
438		min	-.007	2	-.023	3	-.004	3	-1.144e-4	4	985.522	2	2876.146	4
439	11	max	.006	3	.026	2	.024	5	-1.603e-7	10	NC	5	NC	1
440		min	-.007	2	-.023	3	-.004	3	-1.275e-4	4	987.987	2	2330.192	4
441	12	max	.006	3	.024	2	.028	5	-1.078e-7	10	NC	5	NC	1



Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499		3	max	0	9	.064	3	.011	4	5.929e-3	2	NC	5	NC	2
500			min	-.057	4	-.118	2	-.004	10	-4.099e-3	3	1084.587	2	9009.466	1
501		4	max	0	9	.08	3	.013	14	6.911e-3	2	NC	5	NC	2
502			min	-.057	4	-.147	2	-.005	10	-4.754e-3	3	828.79	2	6469.217	1
503		5	max	0	9	.087	3	.015	3	7.892e-3	2	NC	5	NC	2
504			min	-.057	4	-.16	2	-.006	10	-5.41e-3	3	752.857	2	6040.633	1
505		6	max	0	9	.086	3	.017	3	8.873e-3	2	NC	5	NC	2
506			min	-.057	4	-.155	2	-.008	2	-6.065e-3	3	776.16	2	7239.017	1
507		7	max	0	9	.079	3	.018	3	9.854e-3	2	NC	5	NC	1
508			min	-.057	4	-.138	2	-.012	2	-6.721e-3	3	895.717	2	8471.248	3
509		8	max	0	9	.068	3	.018	3	1.084e-2	2	NC	5	NC	1
510			min	-.057	4	-.113	2	-.016	2	-7.376e-3	3	1149.376	2	8102.749	3
511		9	max	0	9	.057	3	.018	3	1.182e-2	2	NC	4	NC	1
512			min	-.057	4	-.089	2	-.02	2	-8.032e-3	3	1574.575	2	7560.118	2
513		10	max	0	9	.052	3	.018	3	1.28e-2	2	NC	4	NC	4
514			min	-.057	4	-.078	2	-.022	2	-8.687e-3	3	1902.457	2	6717.233	2
515		11	max	0	9	.057	3	.016	3	1.182e-2	2	NC	4	NC	1
516			min	-.057	4	-.089	2	-.02	2	-8.031e-3	3	1574.575	2	7560.137	2
517		12	max	0	1	.068	3	.015	3	1.084e-2	2	NC	5	NC	1
518			min	-.057	4	-.113	2	-.016	2	-7.374e-3	3	1149.376	2	NC	1
519		13	max	0	1	.079	3	.014	3	9.855e-3	2	NC	5	NC	1
520			min	-.057	4	-.138	2	-.012	2	-6.717e-3	3	895.717	2	NC	1
521		14	max	0	1	.086	3	.013	3	8.874e-3	2	NC	5	NC	2
522			min	-.057	4	-.155	2	-.008	2	-6.061e-3	3	776.16	2	7248.289	1
523		15	max	0	1	.087	3	.013	1	7.893e-3	2	NC	5	NC	2
524			min	-.057	4	-.16	2	-.006	10	-5.404e-3	3	752.857	2	6056.176	1
525		16	max	0	1	.08	3	.012	1	6.911e-3	2	NC	5	NC	2
526			min	-.057	4	-.147	2	-.005	10	-4.748e-3	3	828.79	2	6494.667	1
527		17	max	0	1	.064	3	.008	3	5.93e-3	2	NC	5	NC	2
528			min	-.057	4	-.118	2	-.004	10	-4.091e-3	3	1084.587	2	9062.113	1
529		18	max	0	1	.042	3	.007	3	4.949e-3	2	NC	4	NC	1
530			min	-.057	4	-.075	2	-.005	2	-3.435e-3	3	1994.183	2	NC	1
531		19	max	0	1	.017	3	.006	3	3.968e-3	2	NC	1	NC	1
532			min	-.057	4	-.024	2	-.007	2	-2.778e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	1	3.492e-4	3	NC	1	NC	1
534			min	0	1	0	1	0	1	-4.99e-4	5	NC	1	NC	1
535		2	max	0	3	0	5	.004	4	7.841e-4	3	NC	1	NC	1
536			min	0	4	-.002	1	0	3	-5.137e-4	5	NC	1	NC	1
537		3	max	0	3	0	5	.008	4	1.219e-3	3	NC	1	NC	1
538			min	0	4	-.004	1	-.003	3	-9.163e-4	2	NC	1	7795.164	4
539		4	max	0	3	.001	5	.012	4	1.654e-3	3	NC	3	NC	9
540			min	-.001	4	-.006	1	-.006	3	-1.35e-3	2	9960.006	1	5121.626	4
541		5	max	0	3	.001	5	.016	4	2.089e-3	3	NC	3	NC	9
542			min	-.002	4	-.008	1	-.01	3	-1.783e-3	2	7771.894	1	3893.002	4
543		6	max	0	3	.002	5	.019	4	2.524e-3	3	NC	5	8872.314	9
544			min	-.002	4	-.009	1	-.015	3	-2.216e-3	2	6540.87	1	2963.815	3
545		7	max	0	3	.002	5	.021	4	2.959e-3	3	NC	5	6957.377	9
546			min	-.003	4	-.011	1	-.019	3	-2.65e-3	2	5800.572	1	2313.319	3
547		8	max	0	3	.002	5	.023	4	3.394e-3	3	NC	5	5750.124	9
548			min	-.003	4	-.011	1	-.024	3	-3.083e-3	2	5356.282	1	1905.107	3
549		9	max	0	3	.003	5	.024	1	3.828e-3	3	NC	5	4958.661	9
550			min	-.004	4	-.012	1	-.028	3	-3.516e-3	2	5117.138	1	1638.268	3
551		10	max	0	3	.003	5	.027	1	4.263e-3	3	NC	5	4992.747	15
552			min	-.004	4	-.012	1	-.031	3	-3.95e-3	2	5041.485	1	1462.225	3
553		11	max	0	3	.003	5	.029	1	4.698e-3	3	NC	5	5529.867	15
554			min	-.005	4	-.012	1	-.034	3	-4.383e-3	2	5117.138	1	1350.455	3
555		12	max	0	3	.003	5	.03	1	5.133e-3	3	NC	5	6516.169	15





Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.