

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	Height = 1550 mm
Width =	1050 mm	Width = 970 mm
Dead Load =	3.00 psf	Dead Load = 1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	99 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.684 k-ft
$M_z$ =	0.106 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>70%</b>

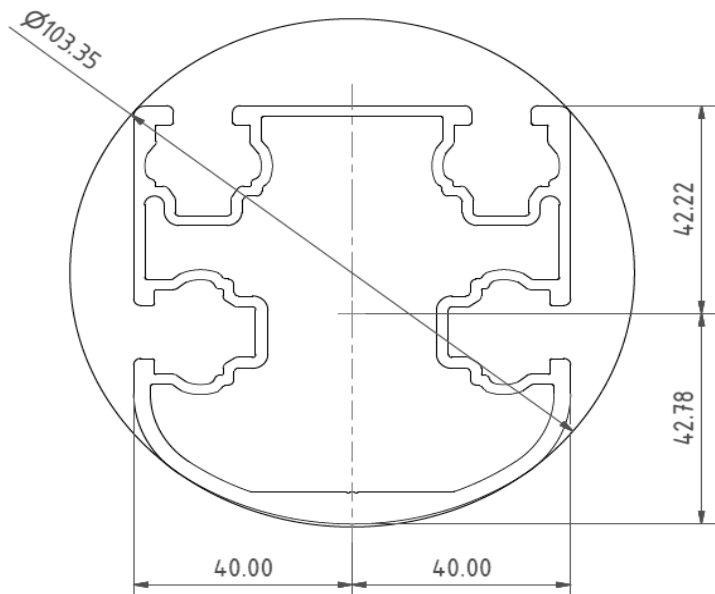


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.761 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.966 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>81%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.993 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>7%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

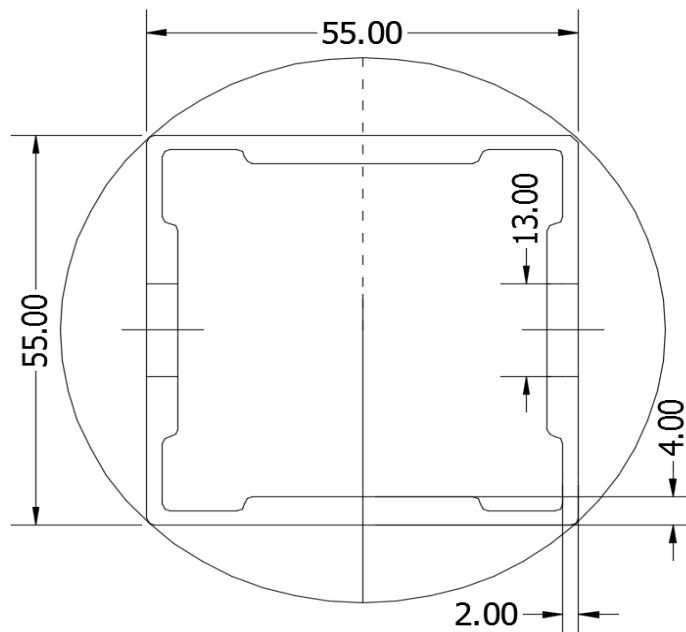
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.821 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>39%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.994 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>35%</b>



#### 5. FOUNDATION DESIGN CALCULATIONS

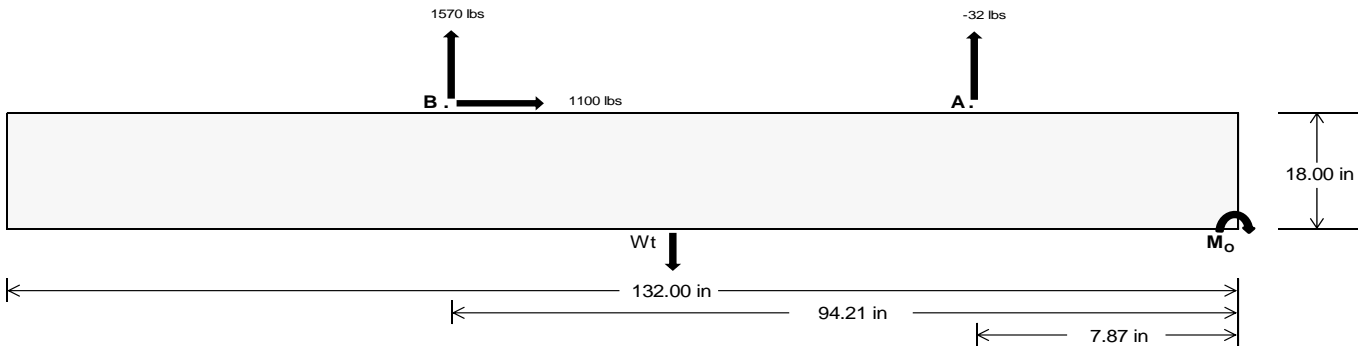
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =		<u>78.71</u>	<u>6540.73</u> k
Compressive Load =		<u>2590.61</u>	<u>4852.49</u> k
Lateral Load =		<u>10.39</u>	<u>4575.55</u> k
Moment (Weak Axis) =		<u>0.02</u>	<u>0.00</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 167496.0$  in-lbs  
Resisting Force Required = 2537.82 lbs  
S.F. = 1.67  
Weight Required = 4229.70 lbs  
Minimum Width = 33 in  
Weight Provided = 6579.38 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 1099.77 lbs  
Friction = 0.4  
Weight Required = 2749.41 lbs  
Resisting Weight = 6579.38 lbs  
Additional Weight Required = 0 lbs

Use a 132in long x 33in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 1099.77 lbs  
Cohesion = 130 psf  
Area = 30.25 ft<sup>2</sup>  
Resisting = 3289.69 lbs  
Additional Weight Required = 0 lbs

Use a 132in long x 33in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.75 \text{ ft}) =$   
33 in 34 in 35 in 36 in  
6579 lbs 6779 lbs 6978 lbs 7178 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in
$F_A$	891 lbs	891 lbs	891 lbs	891 lbs	1043 lbs	1043 lbs	1043 lbs	1043 lbs	1339 lbs	1339 lbs	1339 lbs	1339 lbs	63 lbs	63 lbs	63 lbs	63 lbs
$F_B$	793 lbs	793 lbs	793 lbs	793 lbs	2196 lbs	2196 lbs	2196 lbs	2196 lbs	2142 lbs	2142 lbs	2142 lbs	2142 lbs	-3141 lbs	-3141 lbs	-3141 lbs	-3141 lbs
$F_V$	134 lbs	134 lbs	134 lbs	134 lbs	1995 lbs	1995 lbs	1995 lbs	1995 lbs	1582 lbs	1582 lbs	1582 lbs	1582 lbs	-2200 lbs	-2200 lbs	-2200 lbs	-2200 lbs
$P_{total}$	8263 lbs	8462 lbs	8662 lbs	8861 lbs	9818 lbs	10017 lbs	10217 lbs	10416 lbs	10061 lbs	10261 lbs	10460 lbs	10659 lbs	870 lbs	990 lbs	1109 lbs	1229 lbs
$M$	2654 lbs-ft	2654 lbs-ft	2654 lbs-ft	2654 lbs-ft	2883 lbs-ft	2883 lbs-ft	2883 lbs-ft	2883 lbs-ft	3824 lbs-ft	3824 lbs-ft	3824 lbs-ft	3824 lbs-ft	4391 lbs-ft	4391 lbs-ft	4391 lbs-ft	4391 lbs-ft
$e$	0.32 ft	0.31 ft	0.31 ft	0.30 ft	0.29 ft	0.29 ft	0.28 ft	0.28 ft	0.38 ft	0.37 ft	0.37 ft	0.36 ft	5.05 ft	4.44 ft	3.96 ft	3.57 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	225.3 psf	225.1 psf	224.8 psf	224.6 psf	272.6 psf	271.0 psf	269.4 psf	268.0 psf	263.6 psf	262.3 psf	261.0 psf	259.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	321.0 psf	318.0 psf	315.1 psf	312.4 psf	376.5 psf	371.9 psf	367.5 psf	363.3 psf	401.6 psf	396.1 psf	391.0 psf	386.2 psf	465.2 psf	219.0 psf	164.5 psf	141.7 psf

Maximum Bearing Pressure = 465 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

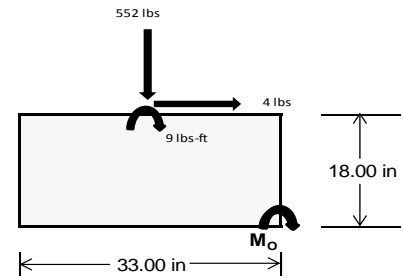
#### Overturning Check

$M_o = 742.9 \text{ ft-lbs}$   
 Resisting Force Required = 540.29 lbs  
 S.F. = 1.67  
 Weight Required = 900.48 lbs  
 Minimum Width = **33 in**  
 Weight Provided = 6579.38 lbs

*A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	33 in			33 in			33 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	220 lbs	519 lbs	220 lbs	552 lbs	1443 lbs	552 lbs	64 lbs	152 lbs	64 lbs
$F_v$	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	8365 lbs	6579 lbs	8365 lbs	8305 lbs	6579 lbs	8305 lbs	2446 lbs	6579 lbs	2446 lbs
$M$	5 lbs-ft	0 lbs-ft	5 lbs-ft	16 lbs-ft	0 lbs-ft	16 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft
$f_{min}$	276.2 psf	217.5 psf	276.2 psf	273.4 psf	217.5 psf	273.4 psf	80.8 psf	217.5 psf	80.8 psf
$f_{max}$	276.9 psf	217.5 psf	276.9 psf	275.7 psf	217.5 psf	275.7 psf	80.9 psf	217.5 psf	80.9 psf



Maximum Bearing Pressure = 277 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 33in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.773 k
Allowable Uplift =	1.214 k
Utilization =	<u>64%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.286 k
Allowable Uplift =	4.357 k
Utilization =	<u>52%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.993 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>27%</u>

#### Rear Strut

Maximum Axial Load =	4.316 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>58%</u>

#### Diagonal Strut

Maximum Axial Load =	2.874 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 99 \text{ in}$$

$$J = 0.432$$

$$273.88$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 99$$

$$J = 0.432$$

$$174.171$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	-63.577	-63.577	0	0
2	M14	y	-63.577	-63.577	0	0
3	M15	y	-105.961	-105.961	0	0
4	M16	y	-105.961	-105.961	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	143.047	143.047	0	0
2	M14	y	111.259	111.259	0	0
3	M15	y	63.577	63.577	0	0
4	M16	y	63.577	63.577	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



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Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	71.353	1	193.272	2	.404	3	.013	2	-.006	15	.83	3
28			min	4.028	15	-333.314	3	-24.521	1	0	3	-.107	1	-.426	2
29		15	max	71.353	1	78.754	2	9.442	1	.013	2	-.006	12	1.041	3
30			min	4.028	15	-127.535	3	.446	10	0	3	-.114	1	-.551	2
31		16	max	71.353	1	78.243	3	43.405	1	.013	2	-.003	12	1.064	3
32			min	4.028	15	-35.765	2	2.451	15	0	3	-.089	1	-.571	2
33		17	max	71.353	1	284.022	3	77.368	1	.013	2	.002	3	.898	3
34			min	4.028	15	-150.283	2	4.33	15	0	3	-.034	1	-.485	2
35		18	max	71.353	1	489.8	3	111.332	1	.013	2	.052	1	.543	3
36			min	4.028	15	-264.801	2	6.209	15	0	3	.003	15	-.295	2
37		19	max	71.353	1	695.579	3	145.295	1	.013	2	.17	1	0	2
38			min	4.028	15	-379.32	2	8.087	15	0	3	.01	15	0	3
39	M14	1	max	37.324	1	421.208	2	-8.38	15	.01	3	.199	1	0	1
40			min	2.101	15	-565.307	3	-150.558	1	-.011	2	.011	15	0	3
41		2	max	37.324	1	306.689	2	-6.501	15	.01	3	.076	1	.445	3
42			min	2.101	15	-406.151	3	-116.595	1	-.011	2	.004	15	-.334	2
43		3	max	37.324	1	192.171	2	-4.623	15	.01	3	.004	3	.745	3
44			min	2.101	15	-246.995	3	-82.632	1	-.011	2	-.015	1	-.562	2
45		4	max	37.324	1	77.653	2	-2.744	15	.01	3	-.002	12	.898	3
46			min	2.101	15	-87.839	3	-48.668	1	-.011	2	-.075	1	-.686	2
47		5	max	37.324	1	71.317	3	-.865	15	.01	3	-.005	12	.906	3
48			min	2.101	15	-36.866	2	-14.705	1	-.011	2	-.104	1	-.705	2
49		6	max	37.324	1	230.473	3	19.258	1	.01	3	-.006	15	.767	3
50			min	2.101	15	-151.384	2	-.841	3	-.011	2	-.102	1	-.618	2
51		7	max	37.324	1	389.629	3	53.221	1	.01	3	-.004	15	.483	3
52			min	2.101	15	-265.902	2	1.414	12	-.011	2	-.069	1	-.427	2
53		8	max	37.324	1	548.785	3	87.185	1	.01	3	.002	10	.053	3
54			min	2.101	15	-380.421	2	3.293	12	-.011	2	-.006	3	-.131	2
55		9	max	37.324	1	707.941	3	121.148	1	.01	3	.091	1	.27	2
56			min	2.101	15	-494.939	2	5.171	12	-.011	2	0	3	-.523	3
57		10	max	37.324	1	609.457	2	-7.049	12	.01	3	.218	1	.777	2
58			min	2.101	15	-867.097	3	-155.111	1	-.011	2	.006	12	-1.245	3
59		11	max	37.324	1	494.939	2	-5.171	12	.011	2	.091	1	.27	2
60			min	2.101	15	-707.941	3	-121.148	1	-.01	3	0	3	-.523	3
61		12	max	37.324	1	380.421	2	-3.293	12	.011	2	.002	10	.053	3
62			min	2.101	15	-548.785	3	-87.185	1	-.01	3	-.006	3	-.131	2
63		13	max	37.324	1	265.902	2	-1.414	12	.011	2	-.004	15	.483	3
64			min	2.101	15	-389.629	3	-53.221	1	-.01	3	-.069	1	-.427	2
65		14	max	37.324	1	151.384	2	.841	3	.011	2	-.006	15	.767	3
66			min	2.101	15	-230.473	3	-19.258	1	-.01	3	-.102	1	-.618	2
67		15	max	37.324	1	36.866	2	14.705	1	.011	2	-.005	12	.906	3
68			min	2.101	15	-71.317	3	.865	15	-.01	3	-.104	1	-.705	2
69		16	max	37.324	1	87.839	3	48.668	1	.011	2	-.002	12	.898	3
70			min	2.101	15	-77.653	2	2.744	15	-.01	3	-.075	1	-.686	2
71		17	max	37.324	1	246.995	3	82.632	1	.011	2	.004	3	.745	3
72			min	2.101	15	-192.171	2	4.623	15	-.01	3	-.015	1	-.562	2
73		18	max	37.324	1	406.151	3	116.595	1	.011	2	.076	1	.445	3
74			min	2.101	15	-306.689	2	6.501	15	-.01	3	.004	15	-.334	2
75		19	max	37.324	1	565.307	3	150.558	1	.011	2	.199	1	0	1
76			min	2.101	15	-421.208	2	8.38	15	-.01	3	.011	15	0	3
77	M15	1	max	-2.193	15	631.042	2	-8.377	15	.012	2	.199	1	0	2
78			min	-38.748	1	-329.511	3	-150.564	1	-.009	3	.011	15	0	3
79		2	max	-2.193	15	454.36	2	-6.499	15	.012	2	.076	1	.261	3
80			min	-38.748	1	-240.289	3	-116.601	1	-.009	3	.004	15	-.497	2
81		3	max	-2.193	15	277.679	2	-4.62	15	.012	2	.004	3	.441	3
82			min	-38.748	1	-151.066	3	-82.638	1	-.009	3	-.015	1	-.833	2
83		4	max	-2.193	15	100.997	2	-2.741	15	.012	2	-.002	12	.538	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-38.748	1	-61.844	3	-48.675	1	-.009	3	-.075	1	-1.007	2
85		5	max	-2.193	15	27.379	3	-.863	15	.012	2	-.005	12	.554	3
86			min	-38.748	1	-75.684	2	-14.711	1	-.009	3	-.104	1	-1.018	2
87		6	max	-2.193	15	116.601	3	19.252	1	.012	2	-.006	15	.488	3
88			min	-38.748	1	-252.366	2	-.646	3	-.009	3	-.102	1	-.868	2
89		7	max	-2.193	15	205.824	3	53.215	1	.012	2	-.004	15	.34	3
90			min	-38.748	1	-429.048	2	1.537	12	-.009	3	-.069	1	-.555	2
91		8	max	-2.193	15	295.046	3	87.178	1	.012	2	.002	10	.111	3
92			min	-38.748	1	-605.729	2	3.415	12	-.009	3	-.006	3	-.081	2
93		9	max	-2.193	15	384.269	3	121.142	1	.012	2	.091	1	.555	2
94			min	-38.748	1	-782.411	2	5.293	12	-.009	3	0	3	-.201	3
95		10	max	-2.193	15	959.092	2	-7.172	12	.012	2	.218	1	1.353	2
96			min	-38.748	1	-473.491	3	-155.105	1	-.009	3	.006	12	-.594	3
97		11	max	-2.193	15	782.411	2	-5.293	12	.009	3	.091	1	.555	2
98			min	-38.748	1	-384.269	3	-121.142	1	-.012	2	0	3	-.201	3
99		12	max	-2.193	15	605.729	2	-3.415	12	.009	3	.002	10	.111	3
100			min	-38.748	1	-295.046	3	-87.178	1	-.012	2	-.006	3	-.081	2
101		13	max	-2.193	15	429.048	2	-1.537	12	.009	3	-.004	15	.34	3
102			min	-38.748	1	-205.824	3	-53.215	1	-.012	2	-.069	1	-.555	2
103		14	max	-2.193	15	252.366	2	.646	3	.009	3	-.006	15	.488	3
104			min	-38.748	1	-116.601	3	-19.252	1	-.012	2	-.102	1	-.868	2
105		15	max	-2.193	15	75.684	2	14.711	1	.009	3	-.005	12	.554	3
106			min	-38.748	1	-27.379	3	.863	15	-.012	2	-.104	1	-1.018	2
107		16	max	-2.193	15	61.844	3	48.675	1	.009	3	-.002	12	.538	3
108			min	-38.748	1	-100.997	2	2.741	15	-.012	2	-.075	1	-1.007	2
109		17	max	-2.193	15	151.066	3	82.638	1	.009	3	.004	3	.441	3
110			min	-38.748	1	-277.679	2	4.62	15	-.012	2	-.015	1	-.833	2
111		18	max	-2.193	15	240.289	3	116.601	1	.009	3	.076	1	.261	3
112			min	-38.748	1	-454.36	2	6.499	15	-.012	2	.004	15	-.497	2
113		19	max	-2.193	15	329.511	3	150.564	1	.009	3	.199	1	0	2
114			min	-38.748	1	-631.042	2	8.377	15	-.012	2	.011	15	0	3
115	M16	1	max	-4.367	15	590.986	2	-8.096	15	.009	2	.172	1	0	2
116			min	-77.492	1	-294.076	3	-145.633	1	-.012	3	.01	15	0	3
117		2	max	-4.367	15	414.305	2	-6.217	15	.009	2	.054	1	.229	3
118			min	-77.492	1	-204.854	3	-111.67	1	-.012	3	.003	15	-.461	2
119		3	max	-4.367	15	237.623	2	-4.339	15	.009	2	0	3	.376	3
120			min	-77.492	1	-115.631	3	-77.707	1	-.012	3	-.033	1	-.76	2
121		4	max	-4.367	15	60.942	2	-2.46	15	.009	2	-.004	12	.441	3
122			min	-77.492	1	-26.409	3	-43.743	1	-.012	3	-.089	1	-.896	2
123		5	max	-4.367	15	62.814	3	-.581	15	.009	2	-.006	12	.424	3
124			min	-77.492	1	-115.74	2	-9.78	1	-.012	3	-.113	1	-.871	2
125		6	max	-4.367	15	152.036	3	24.183	1	.009	2	-.006	15	.326	3
126			min	-77.492	1	-292.421	2	.236	12	-.012	3	-.107	1	-.684	2
127		7	max	-4.367	15	241.259	3	58.146	1	.009	2	-.004	15	.145	3
128			min	-77.492	1	-469.103	2	2.115	12	-.012	3	-.069	1	-.335	2
129		8	max	-4.367	15	330.481	3	92.11	1	.009	2	.003	2	.176	2
130			min	-77.492	1	-645.784	2	3.993	12	-.012	3	-.004	3	-.117	3
131		9	max	-4.367	15	419.704	3	126.073	1	.009	2	.1	1	.849	2
132			min	-77.492	1	-822.466	2	5.871	12	-.012	3	.002	12	-.461	3
133		10	max	-4.367	15	999.148	2	-7.75	12	.009	2	.231	1	1.684	2
134			min	-77.492	1	-508.926	3	-160.036	1	-.012	3	.008	12	-.886	3
135		11	max	-4.367	15	822.466	2	-5.871	12	.012	3	.1	1	.849	2
136			min	-77.492	1	-419.704	3	-126.073	1	-.009	2	.002	12	-.461	3
137		12	max	-4.367	15	645.784	2	-3.993	12	.012	3	.003	2	.176	2
138			min	-77.492	1	-330.481	3	-92.11	1	-.009	2	-.004	3	-.117	3
139		13	max	-4.367	15	469.103	2	-2.115	12	.012	3	-.004	15	.145	3
140			min	-77.492	1	-241.259	3	-58.146	1	-.009	2	-.069	1	-.335	2



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-4.367	15	292.421	2	-.236	12	.012	3	-.006	15	.326	3
142			min	-77.492	1	-152.036	3	-24.183	1	-.009	2	-.107	1	-.684	2
143		15	max	-4.367	15	115.74	2	9.78	1	.012	3	-.006	12	.424	3
144			min	-77.492	1	-62.814	3	.581	15	-.009	2	-.113	1	-.871	2
145		16	max	-4.367	15	26.409	3	43.743	1	.012	3	-.004	12	.441	3
146			min	-77.492	1	-60.942	2	2.46	15	-.009	2	-.089	1	-.896	2
147		17	max	-4.367	15	115.631	3	77.707	1	.012	3	0	3	.376	3
148			min	-77.492	1	-237.623	2	4.339	15	-.009	2	-.033	1	-.76	2
149		18	max	-4.367	15	204.854	3	111.67	1	.012	3	.054	1	.229	3
150			min	-77.492	1	-414.305	2	6.217	15	-.009	2	.003	15	-.461	2
151		19	max	-4.367	15	294.076	3	145.633	1	.012	3	.172	1	0	2
152			min	-77.492	1	-590.986	2	8.096	15	-.009	2	.01	15	0	3
153	M2	1	max	949.793	2	2.019	4	.241	1	0	3	0	3	0	1
154			min	-1345.478	3	.475	15	.014	15	0	1	0	2	0	1
155		2	max	950.314	2	1.9	4	.241	1	0	3	0	1	0	15
156			min	-1345.088	3	.447	15	.014	15	0	1	0	15	0	4
157		3	max	950.835	2	1.781	4	.241	1	0	3	0	1	0	15
158			min	-1344.697	3	.419	15	.014	15	0	1	0	15	-.001	4
159		4	max	951.355	2	1.662	4	.241	1	0	3	0	1	0	15
160			min	-1344.307	3	.391	15	.014	15	0	1	0	15	-.002	4
161		5	max	951.876	2	1.543	4	.241	1	0	3	0	1	0	15
162			min	-1343.916	3	.363	15	.014	15	0	1	0	15	-.003	4
163		6	max	952.397	2	1.424	4	.241	1	0	3	0	1	0	15
164			min	-1343.526	3	.335	15	.014	15	0	1	0	15	-.003	4
165		7	max	952.918	2	1.305	4	.241	1	0	3	0	1	0	15
166			min	-1343.135	3	.307	15	.014	15	0	1	0	15	-.004	4
167		8	max	953.438	2	1.187	4	.241	1	0	3	0	1	0	15
168			min	-1342.745	3	.279	15	.014	15	0	1	0	15	-.004	4
169		9	max	953.959	2	1.068	4	.241	1	0	3	0	1	-.001	15
170			min	-1342.354	3	.247	12	.014	15	0	1	0	15	-.004	4
171		10	max	954.48	2	.949	4	.241	1	0	3	0	1	-.001	15
172			min	-1341.964	3	.2	12	.014	15	0	1	0	15	-.005	4
173		11	max	955	2	.845	2	.241	1	0	3	0	1	-.001	15
174			min	-1341.573	3	.154	12	.014	15	0	1	0	15	-.005	4
175		12	max	955.521	2	.752	2	.241	1	0	3	0	1	-.001	15
176			min	-1341.183	3	.108	12	.014	15	0	1	0	15	-.005	4
177		13	max	956.042	2	.659	2	.241	1	0	3	.001	1	-.001	15
178			min	-1340.792	3	.062	12	.014	15	0	1	0	15	-.006	4
179		14	max	956.562	2	.567	2	.241	1	0	3	.001	1	-.001	15
180			min	-1340.402	3	.006	3	.014	15	0	1	0	15	-.006	4
181		15	max	957.083	2	.474	2	.241	1	0	3	.001	1	-.001	15
182			min	-1340.011	3	-.064	3	.014	15	0	1	0	15	-.006	4
183		16	max	957.604	2	.382	2	.241	1	0	3	.001	1	-.001	15
184			min	-1339.62	3	-.133	3	.014	15	0	1	0	15	-.006	4
185		17	max	958.124	2	.289	2	.241	1	0	3	.001	1	-.001	12
186			min	-1339.23	3	-.203	3	.014	15	0	1	0	15	-.006	4
187		18	max	958.645	2	.196	2	.241	1	0	3	.001	1	-.001	12
188			min	-1338.839	3	-.272	3	.014	15	0	1	0	15	-.006	4
189		19	max	959.166	2	.104	2	.241	1	0	3	.002	1	-.001	12
190			min	-1338.449	3	-.341	3	.014	15	0	1	0	15	-.006	4
191	M3	1	max	817.541	2	7.662	4	.232	1	0	3	0	1	.006	4
192			min	-928.33	3	1.801	15	.013	15	0	1	0	15	.001	12
193		2	max	817.37	2	6.901	4	.232	1	0	3	0	1	.004	2
194			min	-928.458	3	1.622	15	.013	15	0	1	0	15	0	3
195		3	max	817.2	2	6.14	4	.232	1	0	3	0	1	.001	2
196			min	-928.586	3	1.444	15	.013	15	0	1	0	15	-.001	3
197		4	max	817.03	2	5.379	4	.232	1	0	3	0	1	0	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-928.714	3	1.265	15	.013	15	0	1	0	15	-.003	3
199		5	max	816.859	2	4.618	4	.232	1	0	3	0	1	0	15
200			min	-928.841	3	1.086	15	.013	15	0	1	0	15	-.004	4
201		6	max	816.689	2	3.857	4	.232	1	0	3	0	1	-.001	15
202			min	-928.969	3	.907	15	.013	15	0	1	0	15	-.006	4
203		7	max	816.518	2	3.096	4	.232	1	0	3	0	1	-.002	15
204			min	-929.097	3	.728	15	.013	15	0	1	0	15	-.007	4
205		8	max	816.348	2	2.335	4	.232	1	0	3	0	1	-.002	15
206			min	-929.225	3	.549	15	.013	15	0	1	0	15	-.008	4
207		9	max	816.178	2	1.574	4	.232	1	0	3	.001	1	-.002	15
208			min	-929.352	3	.37	15	.013	15	0	1	0	15	-.009	4
209		10	max	816.007	2	.813	4	.232	1	0	3	.001	1	-.002	15
210			min	-929.48	3	.172	12	.013	15	0	1	0	15	-.01	4
211		11	max	815.837	2	.211	2	.232	1	0	3	.001	1	-.002	15
212			min	-929.608	3	-.202	3	.013	15	0	1	0	15	-.01	4
213		12	max	815.667	2	-.166	15	.232	1	0	3	.001	1	-.002	15
214			min	-929.736	3	-.709	4	.013	15	0	1	0	15	-.01	4
215		13	max	815.496	2	-.345	15	.232	1	0	3	.001	1	-.002	15
216			min	-929.863	3	-1.47	4	.013	15	0	1	0	15	-.009	4
217		14	max	815.326	2	-.524	15	.232	1	0	3	.002	1	-.002	15
218			min	-929.991	3	-2.231	4	.013	15	0	1	0	15	-.009	4
219		15	max	815.156	2	-.703	15	.232	1	0	3	.002	1	-.002	15
220			min	-930.119	3	-2.992	4	.013	15	0	1	0	15	-.008	4
221		16	max	814.985	2	-.882	15	.232	1	0	3	.002	1	-.001	15
222			min	-930.247	3	-3.752	4	.013	15	0	1	0	15	-.006	4
223		17	max	814.815	2	-1.061	15	.232	1	0	3	.002	1	-.001	15
224			min	-930.374	3	-4.513	4	.013	15	0	1	0	15	-.004	4
225		18	max	814.645	2	-1.24	15	.232	1	0	3	.002	1	0	15
226			min	-930.502	3	-5.274	4	.013	15	0	1	0	15	-.002	4
227		19	max	814.474	2	-1.419	15	.232	1	0	3	.002	1	0	1
228			min	-930.63	3	-6.035	4	.013	15	0	1	0	15	0	1
229	M4	1	max	795.869	1	0	1	-.459	15	0	1	.002	1	0	1
230			min	38.216	15	0	1	-8.167	1	0	1	0	15	0	1
231		2	max	796.039	1	0	1	-.459	15	0	1	0	1	0	1
232			min	38.267	15	0	1	-8.167	1	0	1	0	15	0	1
233		3	max	796.209	1	0	1	-.459	15	0	1	0	1	0	1
234			min	38.319	15	0	1	-8.167	1	0	1	0	10	0	1
235		4	max	796.38	1	0	1	-.459	15	0	1	0	15	0	1
236			min	38.37	15	0	1	-8.167	1	0	1	0	1	0	1
237		5	max	796.55	1	0	1	-.459	15	0	1	0	15	0	1
238			min	38.421	15	0	1	-8.167	1	0	1	-.002	1	0	1
239		6	max	796.72	1	0	1	-.459	15	0	1	0	15	0	1
240			min	38.473	15	0	1	-8.167	1	0	1	-.003	1	0	1
241		7	max	796.891	1	0	1	-.459	15	0	1	0	15	0	1
242			min	38.524	15	0	1	-8.167	1	0	1	-.004	1	0	1
243		8	max	797.061	1	0	1	-.459	15	0	1	0	15	0	1
244			min	38.575	15	0	1	-8.167	1	0	1	-.005	1	0	1
245		9	max	797.231	1	0	1	-.459	15	0	1	0	15	0	1
246			min	38.627	15	0	1	-8.167	1	0	1	-.006	1	0	1
247		10	max	797.402	1	0	1	-.459	15	0	1	0	15	0	1
248			min	38.678	15	0	1	-8.167	1	0	1	-.007	1	0	1
249		11	max	797.572	1	0	1	-.459	15	0	1	0	15	0	1
250			min	38.73	15	0	1	-8.167	1	0	1	-.007	1	0	1
251		12	max	797.742	1	0	1	-.459	15	0	1	0	15	0	1
252			min	38.781	15	0	1	-8.167	1	0	1	-.008	1	0	1
253		13	max	797.913	1	0	1	-.459	15	0	1	0	15	0	1
254			min	38.832	15	0	1	-8.167	1	0	1	-.009	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	798.083	1	0	1	-459	15	0	1	0	15	0	1
256		min	38.884	15	0	1	-8.167	1	0	1	-.01	1	0	1
257	15	max	798.253	1	0	1	-459	15	0	1	0	15	0	1
258		min	38.935	15	0	1	-8.167	1	0	1	-.011	1	0	1
259	16	max	798.424	1	0	1	-459	15	0	1	0	15	0	1
260		min	38.987	15	0	1	-8.167	1	0	1	-.012	1	0	1
261	17	max	798.594	1	0	1	-459	15	0	1	0	15	0	1
262		min	39.038	15	0	1	-8.167	1	0	1	-.013	1	0	1
263	18	max	798.764	1	0	1	-459	15	0	1	0	15	0	1
264		min	39.089	15	0	1	-8.167	1	0	1	-.014	1	0	1
265	19	max	798.935	1	0	1	-459	15	0	1	0	15	0	1
266		min	39.141	15	0	1	-8.167	1	0	1	-.015	1	0	1
267	M6	1	max	2984.304	2	2.235	2	0	1	0	0	1	0	1
268		min	-4316.46	3	.254	12	0	1	0	1	0	1	0	1
269	2	max	2984.825	2	2.142	2	0	1	0	1	0	1	0	12
270		min	-4316.069	3	.207	12	0	1	0	1	0	1	0	2
271	3	max	2985.346	2	2.05	2	0	1	0	1	0	1	0	12
272		min	-4315.679	3	.161	12	0	1	0	1	0	1	-.002	2
273	4	max	2985.867	2	1.957	2	0	1	0	1	0	1	0	12
274		min	-4315.288	3	.115	12	0	1	0	1	0	1	-.002	2
275	5	max	2986.387	2	1.864	2	0	1	0	1	0	1	0	12
276		min	-4314.898	3	.049	3	0	1	0	1	0	1	-.003	2
277	6	max	2986.908	2	1.772	2	0	1	0	1	0	1	0	12
278		min	-4314.507	3	-.02	3	0	1	0	1	0	1	-.004	2
279	7	max	2987.429	2	1.679	2	0	1	0	1	0	1	0	12
280		min	-4314.117	3	-.09	3	0	1	0	1	0	1	-.004	2
281	8	max	2987.949	2	1.587	2	0	1	0	1	0	1	0	3
282		min	-4313.726	3	-.159	3	0	1	0	1	0	1	-.005	2
283	9	max	2988.47	2	1.494	2	0	1	0	1	0	1	0	3
284		min	-4313.336	3	-.229	3	0	1	0	1	0	1	-.005	2
285	10	max	2988.991	2	1.401	2	0	1	0	1	0	1	0	3
286		min	-4312.945	3	-.298	3	0	1	0	1	0	1	-.006	2
287	11	max	2989.511	2	1.309	2	0	1	0	1	0	1	0	3
288		min	-4312.555	3	-.368	3	0	1	0	1	0	1	-.006	2
289	12	max	2990.032	2	1.216	2	0	1	0	1	0	1	0	3
290		min	-4312.164	3	-.437	3	0	1	0	1	0	1	-.007	2
291	13	max	2990.553	2	1.123	2	0	1	0	1	0	1	0	3
292		min	-4311.773	3	-.507	3	0	1	0	1	0	1	-.007	2
293	14	max	2991.073	2	1.031	2	0	1	0	1	0	1	0	3
294		min	-4311.383	3	-.576	3	0	1	0	1	0	1	-.008	2
295	15	max	2991.594	2	.938	2	0	1	0	1	0	1	0	3
296		min	-4310.992	3	-.646	3	0	1	0	1	0	1	-.008	2
297	16	max	2992.115	2	.846	2	0	1	0	1	0	1	.001	3
298		min	-4310.602	3	-.715	3	0	1	0	1	0	1	-.008	2
299	17	max	2992.636	2	.753	2	0	1	0	1	0	1	.001	3
300		min	-4310.211	3	-.785	3	0	1	0	1	0	1	-.009	2
301	18	max	2993.156	2	.66	2	0	1	0	1	0	1	.002	3
302		min	-4309.821	3	-.854	3	0	1	0	1	0	1	-.009	2
303	19	max	2993.677	2	.568	2	0	1	0	1	0	1	.002	3
304		min	-4309.43	3	-.923	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	2820.562	2	7.685	4	0	1	0	0	1	.009	2
306		min	-2871.931	3	1.805	15	0	1	0	1	0	1	-.002	3
307	2	max	2820.391	2	6.924	4	0	1	0	1	0	1	.006	2
308		min	-2872.059	3	1.626	15	0	1	0	1	0	1	-.003	3
309	3	max	2820.221	2	6.163	4	0	1	0	1	0	1	.004	2
310		min	-2872.187	3	1.447	15	0	1	0	1	0	1	-.005	3
311	4	max	2820.051	2	5.402	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2872.315	3	1.268	15	0	1	0	1	0	1	-.006	3
313	5	max	2819.88	2	4.641	4	0	1	0	1	0	1	0	2
314		min	-2872.442	3	1.09	15	0	1	0	1	0	1	-.007	3
315	6	max	2819.71	2	3.88	4	0	1	0	1	0	1	-.001	15
316		min	-2872.57	3	.911	15	0	1	0	1	0	1	-.007	3
317	7	max	2819.54	2	3.119	4	0	1	0	1	0	1	-.002	15
318		min	-2872.698	3	.732	15	0	1	0	1	0	1	-.008	3
319	8	max	2819.369	2	2.387	2	0	1	0	1	0	1	-.002	15
320		min	-2872.826	3	.453	12	0	1	0	1	0	1	-.008	4
321	9	max	2819.199	2	1.794	2	0	1	0	1	0	1	-.002	15
322		min	-2872.953	3	.157	12	0	1	0	1	0	1	-.009	4
323	10	max	2819.029	2	1.201	2	0	1	0	1	0	1	-.002	15
324		min	-2873.081	3	-.256	3	0	1	0	1	0	1	-.01	4
325	11	max	2818.858	2	.608	2	0	1	0	1	0	1	-.002	15
326		min	-2873.209	3	-.7	3	0	1	0	1	0	1	-.01	4
327	12	max	2818.688	2	.015	2	0	1	0	1	0	1	-.002	15
328		min	-2873.337	3	-1.145	3	0	1	0	1	0	1	-.01	4
329	13	max	2818.518	2	-.341	15	0	1	0	1	0	1	-.002	15
330		min	-2873.465	3	-1.59	3	0	1	0	1	0	1	-.009	4
331	14	max	2818.347	2	-.52	15	0	1	0	1	0	1	-.002	15
332		min	-2873.592	3	-2.208	4	0	1	0	1	0	1	-.009	4
333	15	max	2818.177	2	-.699	15	0	1	0	1	0	1	-.002	15
334		min	-2873.72	3	-2.969	4	0	1	0	1	0	1	-.007	4
335	16	max	2818.007	2	-.878	15	0	1	0	1	0	1	-.001	15
336		min	-2873.848	3	-3.73	4	0	1	0	1	0	1	-.006	4
337	17	max	2817.836	2	-1.057	15	0	1	0	1	0	1	-.001	15
338		min	-2873.976	3	-4.491	4	0	1	0	1	0	1	-.004	4
339	18	max	2817.666	2	-1.236	15	0	1	0	1	0	1	0	15
340		min	-2874.103	3	-5.252	4	0	1	0	1	0	1	-.002	4
341	19	max	2817.496	2	-1.415	15	0	1	0	1	0	1	0	1
342		min	-2874.231	3	-6.013	4	0	1	0	1	0	1	0	1
343	M8	1	max	1989.708	1	0	1	0	1	0	1	0	1	1
344		min	79.074	15	0	1	0	1	0	1	0	1	0	1
345	2	max	1989.879	1	0	1	0	1	0	1	0	1	0	1
346		min	79.125	15	0	1	0	1	0	1	0	1	0	1
347	3	max	1990.049	1	0	1	0	1	0	1	0	1	0	1
348		min	79.177	15	0	1	0	1	0	1	0	1	0	1
349	4	max	1990.219	1	0	1	0	1	0	1	0	1	0	1
350		min	79.228	15	0	1	0	1	0	1	0	1	0	1
351	5	max	1990.39	1	0	1	0	1	0	1	0	1	0	1
352		min	79.28	15	0	1	0	1	0	1	0	1	0	1
353	6	max	1990.56	1	0	1	0	1	0	1	0	1	0	1
354		min	79.331	15	0	1	0	1	0	1	0	1	0	1
355	7	max	1990.73	1	0	1	0	1	0	1	0	1	0	1
356		min	79.382	15	0	1	0	1	0	1	0	1	0	1
357	8	max	1990.901	1	0	1	0	1	0	1	0	1	0	1
358		min	79.434	15	0	1	0	1	0	1	0	1	0	1
359	9	max	1991.071	1	0	1	0	1	0	1	0	1	0	1
360		min	79.485	15	0	1	0	1	0	1	0	1	0	1
361	10	max	1991.241	1	0	1	0	1	0	1	0	1	0	1
362		min	79.537	15	0	1	0	1	0	1	0	1	0	1
363	11	max	1991.412	1	0	1	0	1	0	1	0	1	0	1
364		min	79.588	15	0	1	0	1	0	1	0	1	0	1
365	12	max	1991.582	1	0	1	0	1	0	1	0	1	0	1
366		min	79.639	15	0	1	0	1	0	1	0	1	0	1
367	13	max	1991.752	1	0	1	0	1	0	1	0	1	0	1
368		min	79.691	15	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	1991.923	1	0	1	0	1	0	1	0	1	0	1
370			min	79.742	15	0	1	0	1	0	1	0	1	0	1
371		15	max	1992.093	1	0	1	0	1	0	1	0	1	0	1
372			min	79.794	15	0	1	0	1	0	1	0	1	0	1
373		16	max	1992.263	1	0	1	0	1	0	1	0	1	0	1
374			min	79.845	15	0	1	0	1	0	1	0	1	0	1
375		17	max	1992.434	1	0	1	0	1	0	1	0	1	0	1
376			min	79.896	15	0	1	0	1	0	1	0	1	0	1
377		18	max	1992.604	1	0	1	0	1	0	1	0	1	0	1
378			min	79.948	15	0	1	0	1	0	1	0	1	0	1
379		19	max	1992.775	1	0	1	0	1	0	1	0	1	0	1
380			min	79.999	15	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	949.793	2	2.019	4	-.014	15	0	1	0	2	0	1
382			min	-1345.478	3	.475	15	-.241	1	0	3	0	3	0	1
383		2	max	950.314	2	1.9	4	-.014	15	0	1	0	15	0	15
384			min	-1345.088	3	.447	15	-.241	1	0	3	0	1	0	4
385		3	max	950.835	2	1.781	4	-.014	15	0	1	0	15	0	15
386			min	-1344.697	3	.419	15	-.241	1	0	3	0	1	-.001	4
387		4	max	951.355	2	1.662	4	-.014	15	0	1	0	15	0	15
388			min	-1344.307	3	.391	15	-.241	1	0	3	0	1	-.002	4
389		5	max	951.876	2	1.543	4	-.014	15	0	1	0	15	0	15
390			min	-1343.916	3	.363	15	-.241	1	0	3	0	1	-.003	4
391		6	max	952.397	2	1.424	4	-.014	15	0	1	0	15	0	15
392			min	-1343.526	3	.335	15	-.241	1	0	3	0	1	-.003	4
393		7	max	952.918	2	1.305	4	-.014	15	0	1	0	15	0	15
394			min	-1343.135	3	.307	15	-.241	1	0	3	0	1	-.004	4
395		8	max	953.438	2	1.187	4	-.014	15	0	1	0	15	0	15
396			min	-1342.745	3	.279	15	-.241	1	0	3	0	1	-.004	4
397		9	max	953.959	2	1.068	4	-.014	15	0	1	0	15	-.001	15
398			min	-1342.354	3	.247	12	-.241	1	0	3	0	1	-.004	4
399		10	max	954.48	2	.949	4	-.014	15	0	1	0	15	-.001	15
400			min	-1341.964	3	.2	12	-.241	1	0	3	0	1	-.005	4
401		11	max	955	2	.845	2	-.014	15	0	1	0	15	-.001	15
402			min	-1341.573	3	.154	12	-.241	1	0	3	0	1	-.005	4
403		12	max	955.521	2	.752	2	-.014	15	0	1	0	15	-.001	15
404			min	-1341.183	3	.108	12	-.241	1	0	3	0	1	-.005	4
405		13	max	956.042	2	.659	2	-.014	15	0	1	0	15	-.001	15
406			min	-1340.792	3	.062	12	-.241	1	0	3	-.001	1	-.006	4
407		14	max	956.562	2	.567	2	-.014	15	0	1	0	15	-.001	15
408			min	-1340.402	3	.006	3	-.241	1	0	3	-.001	1	-.006	4
409		15	max	957.083	2	.474	2	-.014	15	0	1	0	15	-.001	15
410			min	-1340.011	3	-.064	3	-.241	1	0	3	-.001	1	-.006	4
411		16	max	957.604	2	.382	2	-.014	15	0	1	0	15	-.001	15
412			min	-1339.62	3	-.133	3	-.241	1	0	3	-.001	1	-.006	4
413		17	max	958.124	2	.289	2	-.014	15	0	1	0	15	-.001	12
414			min	-1339.23	3	-.203	3	-.241	1	0	3	-.001	1	-.006	4
415		18	max	958.645	2	.196	2	-.014	15	0	1	0	15	-.001	12
416			min	-1338.839	3	-.272	3	-.241	1	0	3	-.001	1	-.006	4
417		19	max	959.166	2	.104	2	-.014	15	0	1	0	15	-.001	12
418			min	-1338.449	3	-.341	3	-.241	1	0	3	-.002	1	-.006	4
419	M11	1	max	817.541	2	7.662	4	-.013	15	0	1	0	15	.006	4
420			min	-928.33	3	1.801	15	-.232	1	0	3	0	1	.001	12
421		2	max	817.37	2	6.901	4	-.013	15	0	1	0	15	.004	2
422			min	-928.458	3	1.622	15	-.232	1	0	3	0	1	0	3
423		3	max	817.2	2	6.14	4	-.013	15	0	1	0	15	.001	2
424			min	-928.586	3	1.444	15	-.232	1	0	3	0	1	-.001	3
425		4	max	817.03	2	5.379	4	-.013	15	0	1	0	15	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-928.714	3	1.265	15	-.232	1	0	3	0	1	-.003	3
427		5	max	816.859	2	4.618	4	-.013	15	0	1	0	15	0	15
428			min	-928.841	3	1.086	15	-.232	1	0	3	0	1	-.004	4
429		6	max	816.689	2	3.857	4	-.013	15	0	1	0	15	-.001	15
430			min	-928.969	3	.907	15	-.232	1	0	3	0	1	-.006	4
431		7	max	816.518	2	3.096	4	-.013	15	0	1	0	15	-.002	15
432			min	-929.097	3	.728	15	-.232	1	0	3	0	1	-.007	4
433		8	max	816.348	2	2.335	4	-.013	15	0	1	0	15	-.002	15
434			min	-929.225	3	.549	15	-.232	1	0	3	0	1	-.008	4
435		9	max	816.178	2	1.574	4	-.013	15	0	1	0	15	-.002	15
436			min	-929.352	3	.37	15	-.232	1	0	3	-.001	1	-.009	4
437		10	max	816.007	2	.813	4	-.013	15	0	1	0	15	-.002	15
438			min	-929.48	3	.172	12	-.232	1	0	3	-.001	1	-.01	4
439		11	max	815.837	2	.211	2	-.013	15	0	1	0	15	-.002	15
440			min	-929.608	3	-.202	3	-.232	1	0	3	-.001	1	-.01	4
441		12	max	815.667	2	-.166	15	-.013	15	0	1	0	15	-.002	15
442			min	-929.736	3	-.709	4	-.232	1	0	3	-.001	1	-.01	4
443		13	max	815.496	2	-.345	15	-.013	15	0	1	0	15	-.002	15
444			min	-929.863	3	-1.47	4	-.232	1	0	3	-.001	1	-.009	4
445		14	max	815.326	2	-.524	15	-.013	15	0	1	0	15	-.002	15
446			min	-929.991	3	-2.231	4	-.232	1	0	3	-.002	1	-.009	4
447		15	max	815.156	2	-.703	15	-.013	15	0	1	0	15	-.002	15
448			min	-930.119	3	-2.992	4	-.232	1	0	3	-.002	1	-.008	4
449		16	max	814.985	2	-.882	15	-.013	15	0	1	0	15	-.001	15
450			min	-930.247	3	-3.752	4	-.232	1	0	3	-.002	1	-.006	4
451		17	max	814.815	2	-1.061	15	-.013	15	0	1	0	15	-.001	15
452			min	-930.374	3	-4.513	4	-.232	1	0	3	-.002	1	-.004	4
453		18	max	814.645	2	-1.24	15	-.013	15	0	1	0	15	0	15
454			min	-930.502	3	-5.274	4	-.232	1	0	3	-.002	1	-.002	4
455		19	max	814.474	2	-1.419	15	-.013	15	0	1	0	15	0	1
456			min	-930.63	3	-6.035	4	-.232	1	0	3	-.002	1	0	1
457	M12	1	max	795.869	1	0	1	8.167	1	0	1	0	15	0	1
458			min	38.216	15	0	1	.459	15	0	1	-.002	1	0	1
459		2	max	796.039	1	0	1	8.167	1	0	1	0	15	0	1
460			min	38.267	15	0	1	.459	15	0	1	0	1	0	1
461		3	max	796.209	1	0	1	8.167	1	0	1	0	10	0	1
462			min	38.319	15	0	1	.459	15	0	1	0	1	0	1
463		4	max	796.38	1	0	1	8.167	1	0	1	0	1	0	1
464			min	38.37	15	0	1	.459	15	0	1	0	15	0	1
465		5	max	796.55	1	0	1	8.167	1	0	1	.002	1	0	1
466			min	38.421	15	0	1	.459	15	0	1	0	15	0	1
467		6	max	796.72	1	0	1	8.167	1	0	1	.003	1	0	1
468			min	38.473	15	0	1	.459	15	0	1	0	15	0	1
469		7	max	796.891	1	0	1	8.167	1	0	1	.004	1	0	1
470			min	38.524	15	0	1	.459	15	0	1	0	15	0	1
471		8	max	797.061	1	0	1	8.167	1	0	1	.005	1	0	1
472			min	38.575	15	0	1	.459	15	0	1	0	15	0	1
473		9	max	797.231	1	0	1	8.167	1	0	1	.006	1	0	1
474			min	38.627	15	0	1	.459	15	0	1	0	15	0	1
475		10	max	797.402	1	0	1	8.167	1	0	1	.007	1	0	1
476			min	38.678	15	0	1	.459	15	0	1	0	15	0	1
477		11	max	797.572	1	0	1	8.167	1	0	1	.007	1	0	1
478			min	38.73	15	0	1	.459	15	0	1	0	15	0	1
479		12	max	797.742	1	0	1	8.167	1	0	1	.008	1	0	1
480			min	38.781	15	0	1	.459	15	0	1	0	15	0	1
481		13	max	797.913	1	0	1	8.167	1	0	1	.009	1	0	1
482			min	38.832	15	0	1	.459	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	798.083	1	0	1	8.167	1	0	1	.01	1	0	1
484			min	38.884	15	0	1	.459	15	0	1	0	15	0	1
485		15	max	798.253	1	0	1	8.167	1	0	1	.011	1	0	1
486			min	38.935	15	0	1	.459	15	0	1	0	15	0	1
487		16	max	798.424	1	0	1	8.167	1	0	1	.012	1	0	1
488			min	38.987	15	0	1	.459	15	0	1	0	15	0	1
489		17	max	798.594	1	0	1	8.167	1	0	1	.013	1	0	1
490			min	39.038	15	0	1	.459	15	0	1	0	15	0	1
491		18	max	798.764	1	0	1	8.167	1	0	1	.014	1	0	1
492			min	39.089	15	0	1	.459	15	0	1	0	15	0	1
493		19	max	798.935	1	0	1	8.167	1	0	1	.015	1	0	1
494			min	39.141	15	0	1	.459	15	0	1	0	15	0	1
495	M1	1	max	145.3	1	695.529	3	-4.028	15	0	2	.17	1	0	3
496			min	8.087	15	-378.765	2	-71.282	1	0	3	.01	15	-.013	2
497		2	max	146.122	1	694.649	3	-4.028	15	0	2	.132	1	.187	2
498			min	8.335	15	-379.939	2	-71.282	1	0	3	.007	15	-.366	3
499		3	max	583.094	3	477.214	2	-4.015	15	0	3	.095	1	.377	2
500			min	-334.665	2	-530.12	3	-71.139	1	0	2	.005	15	-.718	3
501		4	max	583.71	3	476.041	2	-4.015	15	0	3	.057	1	.126	2
502			min	-333.843	2	-531	3	-71.139	1	0	2	.003	15	-.438	3
503		5	max	584.326	3	474.867	2	-4.015	15	0	3	.02	1	-.003	15
504			min	-333.021	2	-531.88	3	-71.139	1	0	2	.001	15	-.158	3
505		6	max	584.943	3	473.694	2	-4.015	15	0	3	-.001	15	.123	3
506			min	-332.2	2	-532.76	3	-71.139	1	0	2	-.018	1	-.375	2
507		7	max	585.559	3	472.52	2	-4.015	15	0	3	-.003	15	.405	3
508			min	-331.378	2	-533.64	3	-71.139	1	0	2	-.055	1	-.625	2
509		8	max	586.175	3	471.347	2	-4.015	15	0	3	-.005	15	.686	3
510			min	-330.557	2	-534.52	3	-71.139	1	0	2	-.093	1	-.874	2
511		9	max	601.719	3	52.249	2	-6.115	15	0	9	.057	1	.798	3
512			min	-265.173	2	.359	15	-108.493	1	0	3	.003	15	-1.001	2
513		10	max	602.335	3	51.076	2	-6.115	15	0	9	0	15	.781	3
514			min	-264.351	2	.005	15	-108.493	1	0	3	0	1	-1.028	2
515		11	max	602.952	3	49.902	2	-6.115	15	0	9	-.003	15	.764	3
516			min	-263.53	2	-1.441	4	-108.493	1	0	3	-.058	1	-1.055	2
517		12	max	618.302	3	365.156	3	-3.922	15	0	2	.092	1	.668	3
518			min	-198.065	2	-580.502	2	-69.766	1	0	3	.005	15	-.937	2
519		13	max	618.918	3	364.276	3	-3.922	15	0	2	.055	1	.475	3
520			min	-197.243	2	-581.676	2	-69.766	1	0	3	.003	15	-.63	2
521		14	max	619.535	3	363.396	3	-3.922	15	0	2	.018	1	.283	3
522			min	-196.421	2	-582.849	2	-69.766	1	0	3	.001	15	-.323	2
523		15	max	620.151	3	362.516	3	-3.922	15	0	2	-.001	15	.092	3
524			min	-195.6	2	-584.022	2	-69.766	1	0	3	-.019	1	-.03	1
525		16	max	620.767	3	361.636	3	-3.922	15	0	2	-.003	15	.294	2
526			min	-194.778	2	-585.196	2	-69.766	1	0	3	-.055	1	-.099	3
527		17	max	621.383	3	360.756	3	-3.922	15	0	2	-.005	15	.603	2
528			min	-193.957	2	-586.369	2	-69.766	1	0	3	-.092	1	-.29	3
529		18	max	-8.344	15	592.631	2	-4.368	15	0	3	-.007	15	.304	2
530			min	-146.45	1	-293.285	3	-77.561	1	0	2	-.131	1	-.143	3
531		19	max	-8.096	15	591.458	2	-4.368	15	0	3	-.01	15	.012	3
532			min	-145.629	1	-294.165	3	-77.561	1	0	2	-.172	1	-.009	2
533	M5	1	max	320.738	1	2312.746	3	0	1	0	1	0	1	.027	2
534			min	14.684	12	-1299.75	2	0	1	0	1	0	1	0	3
535		2	max	321.56	1	2311.866	3	0	1	0	1	0	1	.713	2
536			min	15.095	12	-1300.923	2	0	1	0	1	0	1	-1.221	3
537		3	max	1839.158	3	1375.557	2	0	1	0	1	0	1	1.367	2
538			min	-1111.191	2	-1637.512	3	0	1	0	1	0	1	-2.393	3
539		4	max	1839.774	3	1374.384	2	0	1	0	1	0	1	.642	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1110.37	2	-1638.392	3	0	1	0	1	0	1	-1.529	3
541		5	max	1840.391	3	1373.21	2	0	1	0	1	0	1	.01	9
542			min	-1109.548	2	-1639.272	3	0	1	0	1	0	1	-.664	3
543		6	max	1841.007	3	1372.037	2	0	1	0	1	0	1	.201	3
544			min	-1108.727	2	-1640.152	3	0	1	0	1	0	1	-.807	2
545		7	max	1841.623	3	1370.863	2	0	1	0	1	0	1	1.067	3
546			min	-1107.905	2	-1641.032	3	0	1	0	1	0	1	-1.531	2
547		8	max	1842.239	3	1369.69	2	0	1	0	1	0	1	1.933	3
548			min	-1107.083	2	-1641.912	3	0	1	0	1	0	1	-2.254	2
549		9	max	1863.714	3	175.981	2	0	1	0	1	0	1	2.219	3
550			min	-967.278	2	.352	15	0	1	0	1	0	1	-2.576	2
551		10	max	1864.33	3	174.808	2	0	1	0	1	0	1	2.154	3
552			min	-966.457	2	-.002	15	0	1	0	1	0	1	-2.669	2
553		11	max	1864.947	3	173.635	2	0	1	0	1	0	1	2.09	3
554			min	-965.635	2	-1.382	4	0	1	0	1	0	1	-2.761	2
555		12	max	1886.809	3	1104.644	3	0	1	0	1	0	1	1.837	3
556			min	-825.993	2	-1725.653	2	0	1	0	1	0	1	-2.476	2
557		13	max	1887.425	3	1103.764	3	0	1	0	1	0	1	1.254	3
558			min	-825.171	2	-1726.826	2	0	1	0	1	0	1	-1.565	2
559		14	max	1888.042	3	1102.884	3	0	1	0	1	0	1	.672	3
560			min	-824.35	2	-1728	2	0	1	0	1	0	1	-.654	2
561		15	max	1888.658	3	1102.004	3	0	1	0	1	0	1	.259	2
562			min	-823.528	2	-1729.173	2	0	1	0	1	0	1	-.002	13
563		16	max	1889.274	3	1101.124	3	0	1	0	1	0	1	1.171	2
564			min	-822.707	2	-1730.346	2	0	1	0	1	0	1	-.491	3
565		17	max	1889.89	3	1100.244	3	0	1	0	1	0	1	2.085	2
566			min	-821.885	2	-1731.52	2	0	1	0	1	0	1	-1.072	3
567		18	max	-15.909	12	2001.74	2	0	1	0	1	0	1	1.073	2
568			min	-320.903	1	-1017.377	3	0	1	0	1	0	1	-.561	3
569		19	max	-15.498	12	2000.566	2	0	1	0	1	0	1	.017	2
570			min	-320.082	1	-1018.257	3	0	1	0	1	0	1	-.024	3
571	M9	1	max	145.3	1	695.529	3	71.282	1	0	3	-.01	15	0	3
572			min	8.087	15	-378.765	2	4.028	15	0	2	-.17	1	-.013	2
573		2	max	146.122	1	694.649	3	71.282	1	0	3	-.007	15	.187	2
574			min	8.335	15	-379.939	2	4.028	15	0	2	-.132	1	-.366	3
575		3	max	583.094	3	477.214	2	71.139	1	0	2	-.005	15	.377	2
576			min	-334.665	2	-530.12	3	4.015	15	0	3	-.095	1	-.718	3
577		4	max	583.71	3	476.041	2	71.139	1	0	2	-.003	15	.126	2
578			min	-333.843	2	-.531	3	4.015	15	0	3	-.057	1	-.438	3
579		5	max	584.326	3	474.867	2	71.139	1	0	2	-.001	15	-.003	15
580			min	-333.021	2	-531.88	3	4.015	15	0	3	-.02	1	-.158	3
581		6	max	584.943	3	473.694	2	71.139	1	0	2	.018	1	.123	3
582			min	-332.2	2	-532.76	3	4.015	15	0	3	.001	15	-.375	2
583		7	max	585.559	3	472.52	2	71.139	1	0	2	.055	1	.405	3
584			min	-331.378	2	-533.64	3	4.015	15	0	3	.003	15	-.625	2
585		8	max	586.175	3	471.347	2	71.139	1	0	2	.093	1	.686	3
586			min	-330.557	2	-534.52	3	4.015	15	0	3	.005	15	-.874	2
587		9	max	601.719	3	52.249	2	108.493	1	0	3	-.003	15	.798	3
588			min	-265.173	2	.359	15	6.115	15	0	9	-.057	1	-1.001	2
589		10	max	602.335	3	51.076	2	108.493	1	0	3	0	1	.781	3
590			min	-264.351	2	.005	15	6.115	15	0	9	0	15	-1.028	2
591		11	max	602.952	3	49.902	2	108.493	1	0	3	.058	1	.764	3
592			min	-263.53	2	-1.441	4	6.115	15	0	9	.003	15	-1.055	2
593		12	max	618.302	3	365.156	3	69.766	1	0	3	-.005	15	.668	3
594			min	-198.065	2	-580.502	2	3.922	15	0	2	-.092	1	-.937	2
595		13	max	618.918	3	364.276	3	69.766	1	0	3	-.003	15	.475	3
596			min	-197.243	2	-581.676	2	3.922	15	0	2	-.055	1	-.63	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	619.535	3	363.396	3	69.766	1	0	3	-.001	15	.283	3
598		min	-196.421	2	-582.849	2	3.922	15	0	2	-.018	1	-.323	2
599	15	max	620.151	3	362.516	3	69.766	1	0	3	.019	1	.092	3
600		min	-195.6	2	-584.022	2	3.922	15	0	2	.001	15	-.03	1
601	16	max	620.767	3	361.636	3	69.766	1	0	3	.055	1	.294	2
602		min	-194.778	2	-585.196	2	3.922	15	0	2	.003	15	-.099	3
603	17	max	621.383	3	360.756	3	69.766	1	0	3	.092	1	.603	2
604		min	-193.957	2	-586.369	2	3.922	15	0	2	.005	15	-.29	3
605	18	max	-8.344	15	592.631	2	77.561	1	0	2	.131	1	.304	2
606		min	-146.45	1	-293.285	3	4.368	15	0	3	.007	15	-.143	3
607	19	max	-8.096	15	591.458	2	77.561	1	0	2	.172	1	.012	3
608		min	-145.629	1	-294.165	3	4.368	15	0	3	.01	15	-.009	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.106	2	.01	3	8.978e-3	2	NC	1	NC	1
2			min	0	15	-.024	3	-.006	2	-2.538e-3	3	NC	1	NC	1
3		2	max	0	1	.162	3	.019	1	1.005e-2	2	NC	4	NC	1
4			min	0	15	0	9	-.002	10	-2.569e-3	3	1061.158	3	NC	1
5		3	max	0	1	.314	3	.046	1	1.113e-2	2	NC	5	NC	2
6			min	0	15	-.046	1	.002	10	-2.599e-3	3	585.801	3	4284.683	1
7		4	max	0	1	.406	3	.068	1	1.22e-2	2	NC	5	NC	3
8			min	0	15	-.078	2	.004	10	-2.63e-3	3	459.843	3	2878.424	1
9		5	max	0	1	.429	3	.079	1	1.327e-2	2	NC	5	NC	3
10			min	0	15	-.074	2	.004	10	-2.66e-3	3	436.831	3	2483.306	1
11		6	max	0	1	.383	3	.075	1	1.434e-2	2	NC	5	NC	3
12			min	0	15	-.042	1	.002	10	-2.691e-3	3	485.823	3	2612.129	1
13		7	max	0	1	.283	3	.057	1	1.542e-2	2	NC	4	NC	2
14			min	0	15	-.003	9	-.002	10	-2.721e-3	3	643.823	3	3418.683	1
15		8	max	0	1	.155	3	.031	3	1.649e-2	2	NC	1	NC	2
16			min	0	15	.002	15	-.007	10	-2.752e-3	3	1100.77	3	6350.196	1
17		9	max	0	1	.184	2	.031	3	1.756e-2	2	NC	4	NC	1
18			min	0	15	.004	15	-.016	2	-2.782e-3	3	2534.932	2	9527.097	3
19		10	max	0	1	.215	2	.03	3	1.864e-2	2	NC	3	NC	1
20			min	0	1	-.013	3	-.021	2	-2.813e-3	3	1808.358	2	9699.899	3
21		11	max	0	15	.184	2	.031	3	1.756e-2	2	NC	4	NC	1
22			min	0	1	.004	15	-.016	2	-2.782e-3	3	2534.932	2	9527.097	3
23		12	max	0	15	.155	3	.031	3	1.649e-2	2	NC	1	NC	2
24			min	0	1	.002	15	-.007	10	-2.752e-3	3	1100.77	3	6350.196	1
25		13	max	0	15	.283	3	.057	1	1.542e-2	2	NC	4	NC	2
26			min	0	1	-.003	9	-.002	10	-2.721e-3	3	643.823	3	3418.683	1
27		14	max	0	15	.383	3	.075	1	1.434e-2	2	NC	5	NC	3
28			min	0	1	-.042	1	.002	10	-2.691e-3	3	485.823	3	2612.129	1
29		15	max	0	15	.429	3	.079	1	1.327e-2	2	NC	5	NC	3
30			min	0	1	-.074	2	.004	10	-2.66e-3	3	436.831	3	2483.306	1
31		16	max	0	15	.406	3	.068	1	1.22e-2	2	NC	5	NC	3
32			min	0	1	-.078	2	.004	10	-2.63e-3	3	459.843	3	2878.424	1
33		17	max	0	15	.314	3	.046	1	1.113e-2	2	NC	5	NC	2
34			min	0	1	-.046	1	.002	10	-2.599e-3	3	585.801	3	4284.683	1
35		18	max	0	15	.162	3	.019	1	1.005e-2	2	NC	4	NC	1
36			min	0	1	0	9	-.002	10	-2.569e-3	3	1061.158	3	NC	1
37		19	max	0	15	.106	2	.01	3	8.978e-3	2	NC	1	NC	1
38			min	0	1	-.024	3	-.006	2	-2.538e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.247	3	.009	3	5.058e-3	2	NC	1	NC	1
40			min	0	15	-.34	2	-.005	2	-4.178e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.458	3	.012	1	5.952e-3	2	NC	5	NC	1
42		min	0	15	-.528	2	-.002	10	-4.992e-3	3	935.115	3	NC	1
43	3	max	0	1	.641	3	.036	1	6.846e-3	2	NC	5	NC	2
44		min	0	15	-.694	2	0	10	-5.806e-3	3	501.775	3	5506.201	1
45	4	max	0	1	.775	3	.057	1	7.74e-3	2	NC	5	NC	3
46		min	0	15	-.824	2	.003	10	-6.62e-3	3	374.56	3	3438.369	1
47	5	max	0	1	.85	3	.069	1	8.633e-3	2	NC	5	NC	3
48		min	0	15	-.908	2	.004	10	-7.434e-3	3	328.049	3	2848.868	1
49	6	max	0	1	.866	3	.068	1	9.527e-3	2	NC	5	NC	3
50		min	0	15	-.946	2	.002	10	-8.249e-3	3	319.903	3	2919.693	1
51	7	max	0	1	.831	3	.053	1	1.042e-2	2	NC	5	NC	2
52		min	0	15	-.943	2	-.002	10	-9.063e-3	3	328.248	2	3749.488	1
53	8	max	0	1	.764	3	.029	1	1.131e-2	2	NC	5	NC	2
54		min	0	15	-.912	2	-.006	10	-9.877e-3	3	345.825	2	6844.64	1
55	9	max	0	1	.695	3	.027	3	1.221e-2	2	NC	5	NC	1
56		min	0	15	-.874	2	-.014	2	-1.069e-2	3	370.965	2	NC	1
57	10	max	0	1	.662	3	.027	3	1.31e-2	2	NC	5	NC	1
58		min	0	1	-.853	2	-.02	2	-1.151e-2	3	385.55	2	NC	1
59	11	max	0	15	.695	3	.027	3	1.221e-2	2	NC	5	NC	1
60		min	0	1	-.874	2	-.014	2	-1.069e-2	3	370.965	2	NC	1
61	12	max	0	15	.764	3	.029	1	1.131e-2	2	NC	5	NC	2
62		min	0	1	-.912	2	-.006	10	-9.877e-3	3	345.825	2	6844.64	1
63	13	max	0	15	.831	3	.053	1	1.042e-2	2	NC	5	NC	2
64		min	0	1	-.943	2	-.002	10	-9.063e-3	3	328.248	2	3749.488	1
65	14	max	0	15	.866	3	.068	1	9.527e-3	2	NC	5	NC	3
66		min	0	1	-.946	2	.002	10	-8.249e-3	3	319.903	3	2919.693	1
67	15	max	0	15	.85	3	.069	1	8.633e-3	2	NC	5	NC	3
68		min	0	1	-.908	2	.004	10	-7.434e-3	3	328.049	3	2848.868	1
69	16	max	0	15	.775	3	.057	1	7.74e-3	2	NC	5	NC	3
70		min	0	1	-.824	2	.003	10	-6.62e-3	3	374.56	3	3438.369	1
71	17	max	0	15	.641	3	.036	1	6.846e-3	2	NC	5	NC	2
72		min	0	1	-.694	2	0	10	-5.806e-3	3	501.775	3	5506.201	1
73	18	max	0	15	.458	3	.012	1	5.952e-3	2	NC	5	NC	1
74		min	0	1	-.528	2	-.002	10	-4.992e-3	3	935.115	3	NC	1
75	19	max	0	15	.247	3	.009	3	5.058e-3	2	NC	1	NC	1
76		min	0	1	-.34	2	-.005	2	-4.178e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.25	.008	3	3.711e-3	3	NC	1	NC	1
78		min	0	1	-.339	2	-.005	2	-5.324e-3	2	NC	1	NC	1
79	2	max	0	15	.4	3	.013	1	4.438e-3	3	NC	5	NC	1
80		min	0	1	-.582	2	-.002	10	-6.271e-3	2	812.624	2	NC	1
81	3	max	0	15	.533	3	.036	1	5.165e-3	3	NC	5	NC	2
82		min	0	1	-.794	2	.001	10	-7.219e-3	2	435.128	2	5484.961	1
83	4	max	0	15	.637	3	.058	1	5.893e-3	3	NC	5	NC	3
84		min	0	1	-.951	2	.003	10	-8.166e-3	2	323.646	2	3426.488	1
85	5	max	0	15	.705	3	.07	1	6.62e-3	3	NC	5	NC	3
86		min	0	1	-1.041	2	.004	10	-9.114e-3	2	281.93	2	2838.538	1
87	6	max	0	15	.737	3	.068	1	7.348e-3	3	NC	5	NC	3
88		min	0	1	-1.065	2	.002	10	-1.006e-2	2	272.789	2	2906.909	1
89	7	max	0	15	.737	3	.053	1	8.075e-3	3	NC	5	NC	2
90		min	0	1	-1.031	2	0	10	-1.101e-2	2	285.96	2	3725.862	1
91	8	max	0	15	.715	3	.029	1	8.802e-3	3	NC	5	NC	2
92		min	0	1	-.962	2	-.006	10	-1.196e-2	2	317.902	2	6760.056	1
93	9	max	0	15	.686	3	.025	3	9.53e-3	3	NC	5	NC	1
94		min	0	1	-.888	2	-.013	2	-1.29e-2	2	360.821	2	NC	1
95	10	max	0	1	.67	3	.025	3	1.026e-2	3	NC	5	NC	1
96		min	0	1	-.851	2	-.019	2	-1.385e-2	2	386.287	2	NC	1
97	11	max	0	1	.686	3	.025	3	9.53e-3	3	NC	5	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-0.888	2	-0.013	2	-1.29e-2	2	360.821	2	NC	1
99		max	0	1	.715	3	.029	1	8.802e-3	3	NC	5	NC	2
100		min	0	15	-.962	2	-.006	10	-1.196e-2	2	317.902	2	6760.056	1
101		max	0	1	.737	3	.053	1	8.075e-3	3	NC	5	NC	2
102		min	0	15	-1.031	2	0	10	-1.101e-2	2	285.96	2	3725.862	1
103		max	0	1	.737	3	.068	1	7.348e-3	3	NC	5	NC	3
104		min	0	15	-1.065	2	.002	10	-1.006e-2	2	272.789	2	2906.909	1
105		max	0	1	.705	3	.07	1	6.62e-3	3	NC	5	NC	3
106		min	0	15	-1.041	2	.004	10	-9.114e-3	2	281.93	2	2838.538	1
107		max	0	1	.637	3	.058	1	5.893e-3	3	NC	5	NC	3
108		min	0	15	-.951	2	.003	10	-8.166e-3	2	323.646	2	3426.488	1
109		max	0	1	.533	3	.036	1	5.165e-3	3	NC	5	NC	2
110		min	0	15	-.794	2	.001	10	-7.219e-3	2	435.128	2	5484.961	1
111		max	0	1	.4	3	.013	1	4.438e-3	3	NC	5	NC	1
112		min	0	15	-.582	2	-.002	10	-6.271e-3	2	812.624	2	NC	1
113		max	0	1	.25	3	.008	3	3.711e-3	3	NC	1	NC	1
114		min	0	15	-.339	2	-.005	2	-5.324e-3	2	NC	1	NC	1
115	M16	max	0	15	.093	2	.007	3	6.66e-3	3	NC	1	NC	1
116		min	0	1	-.082	3	-.005	2	-7.317e-3	2	NC	1	NC	1
117		max	0	15	.003	4	.019	1	7.609e-3	3	NC	4	NC	1
118		min	0	1	-.052	2	0	10	-8.007e-3	2	1367.285	2	NC	1
119		max	0	15	.028	3	.046	1	8.558e-3	3	NC	5	NC	2
120		min	0	1	-.167	2	.003	10	-8.698e-3	2	762.118	2	4285.965	1
121		max	0	15	.051	3	.069	1	9.507e-3	3	NC	5	NC	3
122		min	0	1	-.232	2	.004	15	-9.388e-3	2	609.147	2	2870.611	1
123		max	0	15	.045	3	.08	1	1.046e-2	3	NC	5	NC	3
124		min	0	1	-.238	2	.005	15	-1.008e-2	2	597.437	2	2468.708	1
125		max	0	15	.011	3	.076	1	1.14e-2	3	NC	5	NC	3
126		min	0	1	-.188	2	.004	10	-1.077e-2	2	705.656	2	2584.938	1
127		max	0	15	.003	4	.059	1	1.235e-2	3	NC	4	NC	2
128		min	0	1	-.092	2	0	10	-1.146e-2	2	1070.032	2	3353.636	1
129		max	0	15	.041	1	.032	1	1.33e-2	3	NC	4	NC	2
130		min	0	1	-.108	3	-.004	10	-1.215e-2	2	2909.918	2	6075.852	1
131		max	0	15	.129	2	.022	3	1.425e-2	3	NC	4	NC	1
132		min	0	1	-.164	3	-.011	2	-1.284e-2	2	2411.05	3	NC	1
133		max	0	1	.176	2	.022	3	1.52e-2	3	NC	4	NC	1
134		min	0	1	-.189	3	-.017	2	-1.353e-2	2	1851.13	3	NC	1
135		max	0	1	.129	2	.022	3	1.425e-2	3	NC	4	NC	1
136		min	0	15	-.164	3	-.011	2	-1.284e-2	2	2411.05	3	NC	1
137		max	0	1	.041	1	.032	1	1.33e-2	3	NC	4	NC	2
138		min	0	15	-.108	3	-.004	10	-1.215e-2	2	2909.918	2	6075.852	1
139		max	0	1	.003	4	.059	1	1.235e-2	3	NC	4	NC	2
140		min	0	15	-.092	2	0	10	-1.146e-2	2	1070.032	2	3353.636	1
141		max	0	1	.011	3	.076	1	1.14e-2	3	NC	5	NC	3
142		min	0	15	-.188	2	.004	10	-1.077e-2	2	705.656	2	2584.938	1
143		max	0	1	.045	3	.08	1	1.046e-2	3	NC	5	NC	3
144		min	0	15	-.238	2	.005	15	-1.008e-2	2	597.437	2	2468.708	1
145		max	0	1	.051	3	.069	1	9.507e-3	3	NC	5	NC	3
146		min	0	15	-.232	2	.004	15	-9.388e-3	2	609.147	2	2870.611	1
147		max	0	1	.028	3	.046	1	8.558e-3	3	NC	5	NC	2
148		min	0	15	-.167	2	.003	10	-8.698e-3	2	762.118	2	4285.965	1
149		max	0	1	.003	4	.019	1	7.609e-3	3	NC	4	NC	1
150		min	0	15	-.052	2	0	10	-8.007e-3	2	1367.285	2	NC	1
151		max	0	1	.093	2	.007	3	6.66e-3	3	NC	1	NC	1
152		min	0	15	-.082	3	-.005	2	-7.317e-3	2	NC	1	NC	1
153	M2	max	.007	2	.01	2	.006	1	-8.747e-6	15	NC	1	NC	1
154		min	-.01	3	-.015	3	0	15	-1.547e-4	1	7825.767	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.007	2	.008	2	.005	1	-8.29e-6	15	NC	1	NC	1
156			min	-.009	3	-.015	3	0	15	-1.466e-4	1	9112.374	2	NC	1
157		3	max	.006	2	.007	2	.005	1	-7.833e-6	15	NC	1	NC	1
158			min	-.009	3	-.014	3	0	15	-1.385e-4	1	NC	1	NC	1
159		4	max	.006	2	.006	2	.004	1	-7.376e-6	15	NC	1	NC	1
160			min	-.008	3	-.014	3	0	15	-1.304e-4	1	NC	1	NC	1
161		5	max	.006	2	.004	2	.004	1	-6.919e-6	15	NC	1	NC	1
162			min	-.008	3	-.013	3	0	15	-1.223e-4	1	NC	1	NC	1
163		6	max	.005	2	.003	2	.003	1	-6.463e-6	15	NC	1	NC	1
164			min	-.007	3	-.013	3	0	15	-1.143e-4	1	NC	1	NC	1
165		7	max	.005	2	.002	2	.003	1	-6.006e-6	15	NC	1	NC	1
166			min	-.007	3	-.012	3	0	15	-1.062e-4	1	NC	1	NC	1
167		8	max	.004	2	.001	2	.003	1	-5.549e-6	15	NC	1	NC	1
168			min	-.006	3	-.012	3	0	15	-9.809e-5	1	NC	1	NC	1
169		9	max	.004	2	0	2	.002	1	-5.092e-6	15	NC	1	NC	1
170			min	-.006	3	-.011	3	0	15	-9.e-5	1	NC	1	NC	1
171		10	max	.004	2	0	2	.002	1	-4.635e-6	15	NC	1	NC	1
172			min	-.005	3	-.01	3	0	15	-8.192e-5	1	NC	1	NC	1
173		11	max	.003	2	-.001	2	.001	1	-4.179e-6	15	NC	1	NC	1
174			min	-.004	3	-.009	3	0	15	-7.384e-5	1	NC	1	NC	1
175		12	max	.003	2	-.001	15	.001	1	-3.722e-6	15	NC	1	NC	1
176			min	-.004	3	-.008	3	0	15	-6.575e-5	1	NC	1	NC	1
177		13	max	.002	2	-.001	15	0	1	-3.265e-6	15	NC	1	NC	1
178			min	-.003	3	-.007	3	0	15	-5.767e-5	1	NC	1	NC	1
179		14	max	.002	2	-.001	15	0	1	-2.808e-6	15	NC	1	NC	1
180			min	-.003	3	-.006	3	0	15	-4.959e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-2.352e-6	15	NC	1	NC	1
182			min	-.002	3	-.005	3	0	15	-4.15e-5	1	NC	1	NC	1
183		16	max	.001	2	0	15	0	1	-1.895e-6	15	NC	1	NC	1
184			min	-.002	3	-.004	3	0	15	-3.342e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-1.438e-6	15	NC	1	NC	1
186			min	-.001	3	-.003	4	0	15	-2.533e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-9.812e-7	15	NC	1	NC	1
188			min	0	3	-.001	4	0	15	-1.725e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-5.244e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-9.167e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	1.983e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	1.139e-7	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	1.61e-5	1	NC	1	NC	1
194			min	0	2	-.002	4	0	1	9.055e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	15	3.022e-5	1	NC	1	NC	1
196			min	0	2	-.004	4	0	1	1.697e-6	15	NC	1	NC	1
197		4	max	.001	3	-.001	15	0	15	4.434e-5	1	NC	1	NC	1
198			min	-.001	2	-.006	4	0	1	2.489e-6	15	NC	1	NC	1
199		5	max	.002	3	-.002	15	0	15	5.845e-5	1	NC	1	NC	1
200			min	-.002	2	-.008	4	0	1	3.28e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	1	7.257e-5	1	NC	1	NC	1
202			min	-.002	2	-.01	4	0	3	4.072e-6	15	9252.673	4	NC	1
203		7	max	.003	3	-.003	15	0	1	8.669e-5	1	NC	1	NC	1
204			min	-.002	2	-.011	4	0	3	4.864e-6	15	8001.332	4	NC	1
205		8	max	.003	3	-.003	15	0	1	1.008e-4	1	NC	2	NC	1
206			min	-.003	2	-.013	4	0	12	5.655e-6	15	7231.252	4	NC	1
207		9	max	.004	3	-.003	15	0	1	1.149e-4	1	NC	5	NC	1
208			min	-.003	2	-.014	4	0	15	6.447e-6	15	6781.963	4	NC	1
209		10	max	.004	3	-.003	15	0	1	1.29e-4	1	NC	5	NC	1
210			min	-.004	2	-.014	4	0	15	7.239e-6	15	6576.206	4	NC	1
211		11	max	.004	3	-.003	15	0	1	1.432e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.014	4	0	15	8.03e-6	15	6583.936	4	NC	1
213		max	.005	3	-.003	15	.001	1	1.573e-4	1	NC	5	NC	1
214		min	-.004	2	-.014	4	0	15	8.822e-6	15	6810.791	4	NC	1
215		max	.005	3	-.003	15	.002	1	1.714e-4	1	NC	2	NC	1
216		min	-.005	2	-.013	4	0	15	9.613e-6	15	7301.753	4	NC	1
217		max	.006	3	-.003	15	.002	1	1.855e-4	1	NC	1	NC	1
218		min	-.005	2	-.012	4	0	15	1.041e-5	15	8163.912	4	NC	1
219		max	.006	3	-.002	15	.003	1	1.996e-4	1	NC	1	NC	1
220		min	-.005	2	-.01	4	0	15	1.12e-5	15	9632.542	4	NC	1
221		max	.007	3	-.002	15	.003	1	2.137e-4	1	NC	1	NC	1
222		min	-.006	2	-.008	4	0	15	1.199e-5	15	NC	1	NC	1
223		max	.007	3	-.001	15	.004	1	2.279e-4	1	NC	1	NC	1
224		min	-.006	2	-.006	4	0	15	1.278e-5	15	NC	1	NC	1
225		max	.008	3	0	15	.004	1	2.42e-4	1	NC	1	NC	1
226		min	-.007	2	-.004	3	0	15	1.357e-5	15	NC	1	NC	1
227		max	.008	3	0	10	.005	1	2.561e-4	1	NC	1	NC	1
228		min	-.007	2	-.003	3	0	15	1.436e-5	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	9.642e-5	1	NC	1	NC	2
230		min	0	15	-.008	3	-.005	1	5.428e-6	15	NC	1	4664.192	1
231		max	.002	1	.006	2	0	15	9.642e-5	1	NC	1	NC	2
232		min	0	15	-.008	3	-.005	1	5.428e-6	15	NC	1	5054.775	1
233		max	.002	1	.006	2	0	15	9.642e-5	1	NC	1	NC	2
234		min	0	15	-.007	3	-.004	1	5.428e-6	15	NC	1	5520.75	1
235		max	.002	1	.006	2	0	15	9.642e-5	1	NC	1	NC	2
236		min	0	15	-.007	3	-.004	1	5.428e-6	15	NC	1	6081.506	1
237		max	.001	1	.005	2	0	15	9.642e-5	1	NC	1	NC	2
238		min	0	15	-.007	3	-.004	1	5.428e-6	15	NC	1	6763.408	1
239		max	.001	1	.005	2	0	15	9.642e-5	1	NC	1	NC	2
240		min	0	15	-.006	3	-.003	1	5.428e-6	15	NC	1	7603.024	1
241		max	.001	1	.005	2	0	15	9.642e-5	1	NC	1	NC	2
242		min	0	15	-.006	3	-.003	1	5.428e-6	15	NC	1	8652.259	1
243		max	.001	1	.004	2	0	15	9.642e-5	1	NC	1	NC	2
244		min	0	15	-.005	3	-.002	1	5.428e-6	15	NC	1	9986.8	1
245		max	.001	1	.004	2	0	15	9.642e-5	1	NC	1	NC	1
246		min	0	15	-.005	3	-.002	1	5.428e-6	15	NC	1	NC	1
247		max	0	1	.003	2	0	15	9.642e-5	1	NC	1	NC	1
248		min	0	15	-.004	3	-.002	1	5.428e-6	15	NC	1	NC	1
249		max	0	1	.003	2	0	15	9.642e-5	1	NC	1	NC	1
250		min	0	15	-.004	3	-.001	1	5.428e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	9.642e-5	1	NC	1	NC	1
252		min	0	15	-.003	3	-.001	1	5.428e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	9.642e-5	1	NC	1	NC	1
254		min	0	15	-.003	3	0	1	5.428e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	9.642e-5	1	NC	1	NC	1
256		min	0	15	-.002	3	0	1	5.428e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	9.642e-5	1	NC	1	NC	1
258		min	0	15	-.002	3	0	1	5.428e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	9.642e-5	1	NC	1	NC	1
260		min	0	15	-.001	3	0	1	5.428e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	9.642e-5	1	NC	1	NC	1
262		min	0	15	0	3	0	1	5.428e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	9.642e-5	1	NC	1	NC	1
264		min	0	15	0	3	0	1	5.428e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	9.642e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	5.428e-6	15	NC	1	NC	1
267	M6	max	.022	2	.034	2	0	1	0	1	NC	4	NC	1
268		min	-.032	3	-.048	3	0	1	0	1	1597.513	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.021	2	.031	2	0	1	0	1	NC	4	NC	1
270		min	-.03	3	-.046	3	0	1	0	1	1691.175	3	NC	1
271	3	max	.02	2	.028	2	0	1	0	1	NC	4	NC	1
272		min	-.028	3	-.043	3	0	1	0	1	1796.643	3	NC	1
273	4	max	.018	2	.025	2	0	1	0	1	NC	4	NC	1
274		min	-.027	3	-.04	3	0	1	0	1	1916.421	3	NC	1
275	5	max	.017	2	.022	2	0	1	0	1	NC	4	NC	1
276		min	-.025	3	-.037	3	0	1	0	1	2053.731	3	NC	1
277	6	max	.016	2	.019	2	0	1	0	1	NC	4	NC	1
278		min	-.023	3	-.035	3	0	1	0	1	2212.784	3	NC	1
279	7	max	.015	2	.017	2	0	1	0	1	NC	1	NC	1
280		min	-.021	3	-.032	3	0	1	0	1	2399.199	3	NC	1
281	8	max	.014	2	.014	2	0	1	0	1	NC	1	NC	1
282		min	-.02	3	-.029	3	0	1	0	1	2620.639	3	NC	1
283	9	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
284		min	-.018	3	-.027	3	0	1	0	1	2887.84	3	NC	1
285	10	max	.011	2	.01	2	0	1	0	1	NC	1	NC	1
286		min	-.016	3	-.024	3	0	1	0	1	3216.314	3	NC	1
287	11	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
288		min	-.014	3	-.021	3	0	1	0	1	3629.346	3	NC	1
289	12	max	.009	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.012	3	-.018	3	0	1	0	1	4163.54	3	NC	1
291	13	max	.007	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.011	3	-.016	3	0	1	0	1	4879.934	3	NC	1
293	14	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.009	3	-.013	3	0	1	0	1	5888.437	3	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.007	3	-.01	3	0	1	0	1	7408.928	3	NC	1
297	16	max	.004	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.008	3	0	1	0	1	9954.57	3	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	0	2	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.006	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.007	2	-.012	3	0	1	0	1	8821.234	3	NC	1
317	7	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.014	3	0	1	0	1	7883.123	3	NC	1
319	8	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7323.521	4	NC	1
321	9	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.011	2	-.016	3	0	1	0	1	6863.316	4	NC	1
323	10	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.012	2	-.017	3	0	1	0	1	6650.839	4	NC	1
325	11	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.014	2	-.017	3	0	1	0	1	6655.047	4	NC	1
327		12	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.015	2	-.017	3	0	1	0	1	6881.184	4	NC	1
329		13	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.016	2	-.016	3	0	1	0	1	7374.352	4	NC	1
331		14	max	.018	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.018	2	-.015	3	0	1	0	1	8242.405	4	NC	1
333		15	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.019	2	-.014	3	0	1	0	1	9722.568	4	NC	1
335		16	max	.021	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.02	2	-.013	3	0	1	0	1	NC	1	NC	1
337		17	max	.022	3	0	2	0	1	0	1	NC	1	NC	1
338			min	-.022	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.023	3	0	2	0	1	0	1	NC	1	NC	1
340			min	-.023	2	-.009	3	0	1	0	1	NC	1	NC	1
341		19	max	.025	3	.002	2	0	1	0	1	NC	1	NC	1
342			min	-.024	2	-.008	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.005	1	.024	2	0	1	0	1	NC	1	NC	1
344			min	0	15	-.026	3	0	1	0	1	NC	1	NC	1
345		2	max	.004	1	.023	2	0	1	0	1	NC	1	NC	1
346			min	0	15	-.025	3	0	1	0	1	NC	1	NC	1
347		3	max	.004	1	.021	2	0	1	0	1	NC	1	NC	1
348			min	0	15	-.023	3	0	1	0	1	NC	1	NC	1
349		4	max	.004	1	.02	2	0	1	0	1	NC	1	NC	1
350			min	0	15	-.022	3	0	1	0	1	NC	1	NC	1
351		5	max	.004	1	.019	2	0	1	0	1	NC	1	NC	1
352			min	0	15	-.02	3	0	1	0	1	NC	1	NC	1
353		6	max	.003	1	.017	2	0	1	0	1	NC	1	NC	1
354			min	0	15	-.019	3	0	1	0	1	NC	1	NC	1
355		7	max	.003	1	.016	2	0	1	0	1	NC	1	NC	1
356			min	0	15	-.017	3	0	1	0	1	NC	1	NC	1
357		8	max	.003	1	.015	2	0	1	0	1	NC	1	NC	1
358			min	0	15	-.016	3	0	1	0	1	NC	1	NC	1
359		9	max	.003	1	.013	2	0	1	0	1	NC	1	NC	1
360			min	0	15	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.002	1	.012	2	0	1	0	1	NC	1	NC	1
362			min	0	15	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.002	1	.011	2	0	1	0	1	NC	1	NC	1
364			min	0	15	-.012	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	1	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	15	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.008	2	0	1	0	1	NC	1	NC	1
368			min	0	15	-.009	3	0	1	0	1	NC	1	NC	1
369		14	max	.001	1	.007	2	0	1	0	1	NC	1	NC	1
370			min	0	15	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.001	1	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	15	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	0	1	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	15	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	15	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	15	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.01	2	0	15	1.547e-4	1	NC	1	NC	1
382			min	-.01	3	-.015	3	-.006	1	8.747e-6	15	7825.767	2	NC	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	2	.008	2	0	15	1.466e-4	1	NC	1	NC	1
384		min	-.009	3	-.015	3	-.005	1	8.29e-6	15	9112.374	2	NC	1
385	3	max	.006	2	.007	2	0	15	1.385e-4	1	NC	1	NC	1
386		min	-.009	3	-.014	3	-.005	1	7.833e-6	15	NC	1	NC	1
387	4	max	.006	2	.006	2	0	15	1.304e-4	1	NC	1	NC	1
388		min	-.008	3	-.014	3	-.004	1	7.376e-6	15	NC	1	NC	1
389	5	max	.006	2	.004	2	0	15	1.223e-4	1	NC	1	NC	1
390		min	-.008	3	-.013	3	-.004	1	6.919e-6	15	NC	1	NC	1
391	6	max	.005	2	.003	2	0	15	1.143e-4	1	NC	1	NC	1
392		min	-.007	3	-.013	3	-.003	1	6.463e-6	15	NC	1	NC	1
393	7	max	.005	2	.002	2	0	15	1.062e-4	1	NC	1	NC	1
394		min	-.007	3	-.012	3	-.003	1	6.006e-6	15	NC	1	NC	1
395	8	max	.004	2	.001	2	0	15	9.809e-5	1	NC	1	NC	1
396		min	-.006	3	-.012	3	-.003	1	5.549e-6	15	NC	1	NC	1
397	9	max	.004	2	0	2	0	15	9.e-5	1	NC	1	NC	1
398		min	-.006	3	-.011	3	-.002	1	5.092e-6	15	NC	1	NC	1
399	10	max	.004	2	0	2	0	15	8.192e-5	1	NC	1	NC	1
400		min	-.005	3	-.01	3	-.002	1	4.635e-6	15	NC	1	NC	1
401	11	max	.003	2	-.001	2	0	15	7.384e-5	1	NC	1	NC	1
402		min	-.004	3	-.009	3	-.001	1	4.179e-6	15	NC	1	NC	1
403	12	max	.003	2	-.001	15	0	15	6.575e-5	1	NC	1	NC	1
404		min	-.004	3	-.008	3	-.001	1	3.722e-6	15	NC	1	NC	1
405	13	max	.002	2	-.001	15	0	15	5.767e-5	1	NC	1	NC	1
406		min	-.003	3	-.007	3	0	1	3.265e-6	15	NC	1	NC	1
407	14	max	.002	2	-.001	15	0	15	4.959e-5	1	NC	1	NC	1
408		min	-.003	3	-.006	3	0	1	2.808e-6	15	NC	1	NC	1
409	15	max	.002	2	-.001	15	0	15	4.15e-5	1	NC	1	NC	1
410		min	-.002	3	-.005	3	0	1	2.352e-6	15	NC	1	NC	1
411	16	max	.001	2	0	15	0	15	3.342e-5	1	NC	1	NC	1
412		min	-.002	3	-.004	3	0	1	1.895e-6	15	NC	1	NC	1
413	17	max	0	2	0	15	0	15	2.533e-5	1	NC	1	NC	1
414		min	-.001	3	-.003	4	0	1	1.438e-6	15	NC	1	NC	1
415	18	max	0	2	0	15	0	15	1.725e-5	1	NC	1	NC	1
416		min	0	3	-.001	4	0	1	9.812e-7	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	9.167e-6	1	NC	1	NC	1
418		min	0	1	0	1	0	1	5.244e-7	15	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-1.139e-7	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-1.983e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-9.055e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	15	-1.61e-5	1	NC	1	NC	1
423	3	max	0	3	0	15	0	1	-1.697e-6	15	NC	1	NC	1
424		min	0	2	-.004	4	0	15	-3.022e-5	1	NC	1	NC	1
425	4	max	.001	3	-.001	15	0	1	-2.489e-6	15	NC	1	NC	1
426		min	-.001	2	-.006	4	0	15	-4.434e-5	1	NC	1	NC	1
427	5	max	.002	3	-.002	15	0	1	-3.28e-6	15	NC	1	NC	1
428		min	-.002	2	-.008	4	0	15	-5.845e-5	1	NC	1	NC	1
429	6	max	.002	3	-.002	15	0	3	-4.072e-6	15	NC	1	NC	1
430		min	-.002	2	-.01	4	0	1	-7.257e-5	1	9252.673	4	NC	1
431	7	max	.003	3	-.003	15	0	3	-4.864e-6	15	NC	1	NC	1
432		min	-.002	2	-.011	4	0	1	-8.669e-5	1	8001.332	4	NC	1
433	8	max	.003	3	-.003	15	0	12	-5.655e-6	15	NC	2	NC	1
434		min	-.003	2	-.013	4	0	1	-1.008e-4	1	7231.252	4	NC	1
435	9	max	.004	3	-.003	15	0	15	-6.447e-6	15	NC	5	NC	1
436		min	-.003	2	-.014	4	0	1	-1.149e-4	1	6781.963	4	NC	1
437	10	max	.004	3	-.003	15	0	15	-7.239e-6	15	NC	5	NC	1
438		min	-.004	2	-.014	4	0	1	-1.29e-4	1	6576.206	4	NC	1
439	11	max	.004	3	-.003	15	0	15	-8.03e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.004	2	-.014	4	0	1	-1.432e-4	1	6583.936	4	NC	1
441		max	.005	3	-.003	15	0	15	-8.822e-6	15	NC	5	NC	1
442		min	-.004	2	-.014	4	-.001	1	-1.573e-4	1	6810.791	4	NC	1
443		max	.005	3	-.003	15	0	15	-9.613e-6	15	NC	2	NC	1
444		min	-.005	2	-.013	4	-.002	1	-1.714e-4	1	7301.753	4	NC	1
445		max	.006	3	-.003	15	0	15	-1.041e-5	15	NC	1	NC	1
446		min	-.005	2	-.012	4	-.002	1	-1.855e-4	1	8163.912	4	NC	1
447		max	.006	3	-.002	15	0	15	-1.12e-5	15	NC	1	NC	1
448		min	-.005	2	-.01	4	-.003	1	-1.996e-4	1	9632.542	4	NC	1
449		max	.007	3	-.002	15	0	15	-1.199e-5	15	NC	1	NC	1
450		min	-.006	2	-.008	4	-.003	1	-2.137e-4	1	NC	1	NC	1
451		max	.007	3	-.001	15	0	15	-1.278e-5	15	NC	1	NC	1
452		min	-.006	2	-.006	4	-.004	1	-2.279e-4	1	NC	1	NC	1
453		max	.008	3	0	15	0	15	-1.357e-5	15	NC	1	NC	1
454		min	-.007	2	-.004	3	-.004	1	-2.42e-4	1	NC	1	NC	1
455		max	.008	3	0	10	0	15	-1.436e-5	15	NC	1	NC	1
456		min	-.007	2	-.003	3	-.005	1	-2.561e-4	1	NC	1	NC	1
457	M12	max	.002	1	.007	2	.005	1	-5.428e-6	15	NC	1	NC	2
458		min	0	15	-.008	3	0	15	-9.642e-5	1	NC	1	4664.192	1
459		max	.002	1	.006	2	.005	1	-5.428e-6	15	NC	1	NC	2
460		min	0	15	-.008	3	0	15	-9.642e-5	1	NC	1	5054.775	1
461		max	.002	1	.006	2	.004	1	-5.428e-6	15	NC	1	NC	2
462		min	0	15	-.007	3	0	15	-9.642e-5	1	NC	1	5520.75	1
463		max	.002	1	.006	2	.004	1	-5.428e-6	15	NC	1	NC	2
464		min	0	15	-.007	3	0	15	-9.642e-5	1	NC	1	6081.506	1
465		max	.001	1	.005	2	.004	1	-5.428e-6	15	NC	1	NC	2
466		min	0	15	-.007	3	0	15	-9.642e-5	1	NC	1	6763.408	1
467		max	.001	1	.005	2	.003	1	-5.428e-6	15	NC	1	NC	2
468		min	0	15	-.006	3	0	15	-9.642e-5	1	NC	1	7603.024	1
469		max	.001	1	.005	2	.003	1	-5.428e-6	15	NC	1	NC	2
470		min	0	15	-.006	3	0	15	-9.642e-5	1	NC	1	8652.259	1
471		max	.001	1	.004	2	.002	1	-5.428e-6	15	NC	1	NC	2
472		min	0	15	-.005	3	0	15	-9.642e-5	1	NC	1	9986.8	1
473		max	.001	1	.004	2	.002	1	-5.428e-6	15	NC	1	NC	1
474		min	0	15	-.005	3	0	15	-9.642e-5	1	NC	1	NC	1
475		max	0	1	.003	2	.002	1	-5.428e-6	15	NC	1	NC	1
476		min	0	15	-.004	3	0	15	-9.642e-5	1	NC	1	NC	1
477		max	0	1	.003	2	.001	1	-5.428e-6	15	NC	1	NC	1
478		min	0	15	-.004	3	0	15	-9.642e-5	1	NC	1	NC	1
479		max	0	1	.003	2	.001	1	-5.428e-6	15	NC	1	NC	1
480		min	0	15	-.003	3	0	15	-9.642e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-5.428e-6	15	NC	1	NC	1
482		min	0	15	-.003	3	0	15	-9.642e-5	1	NC	1	NC	1
483		max	0	1	.002	2	0	1	-5.428e-6	15	NC	1	NC	1
484		min	0	15	-.002	3	0	15	-9.642e-5	1	NC	1	NC	1
485		max	0	1	.002	2	0	1	-5.428e-6	15	NC	1	NC	1
486		min	0	15	-.002	3	0	15	-9.642e-5	1	NC	1	NC	1
487		max	0	1	.001	2	0	1	-5.428e-6	15	NC	1	NC	1
488		min	0	15	-.001	3	0	15	-9.642e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-5.428e-6	15	NC	1	NC	1
490		min	0	15	0	3	0	15	-9.642e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-5.428e-6	15	NC	1	NC	1
492		min	0	15	0	3	0	15	-9.642e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-5.428e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-9.642e-5	1	NC	1	NC	1
495	M1	max	.01	3	.106	2	0	1	8.048e-3	2	NC	1	NC	1
496		min	-.006	2	-.024	3	0	15	-1.761e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.01	3	.049	2	0	15	3.945e-3	2	NC	4	NC	1
498		min	-.006	2	-.008	3	-.004	1	-8.714e-3	3	2023.155	2	NC	1
499	3	max	.01	3	.016	3	0	15	2.222e-5	10	NC	5	NC	1
500		min	-.006	2	-.012	2	-.006	1	-1.071e-4	3	978.965	2	NC	1
501	4	max	.01	3	.054	3	0	15	3.244e-3	2	NC	5	NC	1
502		min	-.006	2	-.08	2	-.005	1	-3.749e-3	3	621.53	2	NC	1
503	5	max	.009	3	.1	3	0	15	6.479e-3	2	NC	5	NC	1
504		min	-.006	2	-.15	2	-.004	1	-7.39e-3	3	450.764	2	NC	1
505	6	max	.009	3	.149	3	0	15	9.714e-3	2	NC	15	NC	1
506		min	-.006	2	-.218	2	-.002	1	-1.103e-2	3	356.369	2	NC	1
507	7	max	.009	3	.196	3	0	1	1.295e-2	2	NC	15	NC	1
508		min	-.005	2	-.278	2	0	3	-1.467e-2	3	300.487	2	NC	1
509	8	max	.009	3	.235	3	0	1	1.618e-2	2	NC	15	NC	1
510		min	-.005	2	-.325	2	0	15	-1.831e-2	3	267.361	2	NC	1
511	9	max	.009	3	.26	3	0	15	1.846e-2	2	NC	15	NC	1
512		min	-.005	2	-.356	2	0	1	-1.867e-2	3	250.088	2	NC	1
513	10	max	.008	3	.269	3	0	1	2.01e-2	2	NC	15	NC	1
514		min	-.005	2	-.366	2	0	15	-1.684e-2	3	245.054	2	NC	1
515	11	max	.008	3	.262	3	0	1	2.174e-2	2	NC	15	NC	1
516		min	-.005	2	-.355	2	0	15	-1.502e-2	3	251.088	2	NC	1
517	12	max	.008	3	.24	3	0	15	2.107e-2	2	NC	15	NC	1
518		min	-.005	2	-.324	2	0	1	-1.289e-2	3	270.377	2	NC	1
519	13	max	.008	3	.204	3	0	15	1.69e-2	2	NC	15	NC	1
520		min	-.005	2	-.273	2	0	1	-1.032e-2	3	307.774	2	NC	1
521	14	max	.008	3	.159	3	.001	1	1.273e-2	2	NC	15	NC	1
522		min	-.005	2	-.21	2	0	15	-7.744e-3	3	371.841	2	NC	1
523	15	max	.007	3	.109	3	.003	1	8.557e-3	2	NC	5	NC	1
524		min	-.005	2	-.14	2	0	15	-5.17e-3	3	482.406	2	NC	1
525	16	max	.007	3	.056	3	.005	1	4.387e-3	2	NC	5	NC	1
526		min	-.005	2	-.071	2	0	15	-2.596e-3	3	687.78	2	NC	1
527	17	max	.007	3	.006	3	.005	1	3.651e-4	1	NC	5	NC	1
528		min	-.005	2	-.007	2	0	15	-2.282e-5	3	1128.085	2	NC	1
529	18	max	.007	3	.046	2	.004	1	6.84e-3	2	NC	4	NC	1
530		min	-.005	2	-.039	3	0	15	-2.919e-3	3	2401.088	2	NC	1
531	19	max	.007	3	.093	2	0	15	1.371e-2	2	NC	1	NC	1
532		min	-.005	2	-.082	3	0	1	-5.943e-3	3	NC	1	NC	1
533	M5	1	max	.03	.215	2	0	1	0	1	NC	1	NC	1
534		min	-.021	2	-.013	3	0	1	0	1	NC	1	NC	1
535	2	max	.03	3	.096	2	0	1	0	1	NC	5	NC	1
536		min	-.022	2	.002	15	0	1	0	1	972.16	2	NC	1
537	3	max	.03	3	.051	3	0	1	0	1	NC	5	NC	1
538		min	-.022	2	-.038	2	0	1	0	1	458.138	2	NC	1
539	4	max	.03	3	.14	3	0	1	0	1	NC	15	NC	1
540		min	-.021	2	-.197	2	0	1	0	1	281.046	2	NC	1
541	5	max	.029	3	.259	3	0	1	0	1	9585.122	15	NC	1
542		min	-.021	2	-.369	2	0	1	0	1	198.174	2	NC	1
543	6	max	.028	3	.392	3	0	1	0	1	7368.557	15	NC	1
544		min	-.02	2	-.539	2	0	1	0	1	153.387	2	NC	1
545	7	max	.028	3	.521	3	0	1	0	1	6090.729	15	NC	1
546		min	-.02	2	-.693	2	0	1	0	1	127.367	2	NC	1
547	8	max	.027	3	.629	3	0	1	0	1	5348.716	15	NC	1
548		min	-.02	2	-.816	2	0	1	0	1	112.173	2	NC	1
549	9	max	.027	3	.697	3	0	1	0	1	4968.58	15	NC	1
550		min	-.019	2	-.894	2	0	1	0	1	104.355	2	NC	1
551	10	max	.026	3	.721	3	0	1	0	1	4854.149	15	NC	1
552		min	-.019	2	-.921	2	0	1	0	1	102.082	2	NC	1
553	11	max	.025	3	.702	3	0	1	0	1	4968.902	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.019	2	-.894	2	0	1	0	1	104.803	2	NC	1
555	12	max	.025	3	.641	3	0	1	0	1	5349.458	15	NC	1
556		min	-.018	2	-.811	2	0	1	0	1	113.648	2	NC	1
557	13	max	.024	3	.543	3	0	1	0	1	6092.194	15	NC	1
558		min	-.018	2	-.679	2	0	1	0	1	131.216	2	NC	1
559	14	max	.023	3	.42	3	0	1	0	1	7371.349	15	NC	1
560		min	-.018	2	-.516	2	0	1	0	1	162.11	2	NC	1
561	15	max	.023	3	.284	3	0	1	0	1	9590.539	15	NC	1
562		min	-.018	2	-.34	2	0	1	0	1	217.307	2	NC	1
563	16	max	.022	3	.146	3	0	1	0	1	NC	15	NC	1
564		min	-.017	2	-.168	2	0	1	0	1	324.656	2	NC	1
565	17	max	.022	3	.017	3	0	1	0	1	NC	5	NC	1
566		min	-.017	2	-.021	2	0	1	0	1	566.887	2	NC	1
567	18	max	.022	3	.089	2	0	1	0	1	NC	5	NC	1
568		min	-.017	2	-.091	3	0	1	0	1	1269.706	2	NC	1
569	19	max	.022	3	.176	2	0	1	0	1	NC	1	NC	1
570		min	-.017	2	-.189	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.106	2	0	15	1.761e-2	3	NC	1	NC	1
572		min	-.006	2	-.024	3	0	1	-8.048e-3	2	NC	1	NC	1
573	2	max	.01	3	.049	2	.004	1	8.714e-3	3	NC	4	NC	1
574		min	-.006	2	-.008	3	0	15	-3.945e-3	2	2023.155	2	NC	1
575	3	max	.01	3	.016	3	.006	1	1.071e-4	3	NC	5	NC	1
576		min	-.006	2	-.012	2	0	15	-2.222e-5	10	978.965	2	NC	1
577	4	max	.01	3	.054	3	.005	1	3.749e-3	3	NC	5	NC	1
578		min	-.006	2	-.08	2	0	15	-3.244e-3	2	621.53	2	NC	1
579	5	max	.009	3	.1	3	.004	1	7.39e-3	3	NC	5	NC	1
580		min	-.006	2	-.15	2	0	15	-6.479e-3	2	450.764	2	NC	1
581	6	max	.009	3	.149	3	.002	1	1.103e-2	3	NC	15	NC	1
582		min	-.006	2	-.218	2	0	15	-9.714e-3	2	356.369	2	NC	1
583	7	max	.009	3	.196	3	0	3	1.467e-2	3	NC	15	NC	1
584		min	-.005	2	-.278	2	0	1	-1.295e-2	2	300.487	2	NC	1
585	8	max	.009	3	.235	3	0	15	1.831e-2	3	NC	15	NC	1
586		min	-.005	2	-.325	2	0	1	-1.618e-2	2	267.361	2	NC	1
587	9	max	.009	3	.26	3	0	1	1.867e-2	3	NC	15	NC	1
588		min	-.005	2	-.356	2	0	15	-1.846e-2	2	250.088	2	NC	1
589	10	max	.008	3	.269	3	0	15	1.684e-2	3	NC	15	NC	1
590		min	-.005	2	-.366	2	0	1	-2.01e-2	2	245.054	2	NC	1
591	11	max	.008	3	.262	3	0	15	1.502e-2	3	NC	15	NC	1
592		min	-.005	2	-.355	2	0	1	-2.174e-2	2	251.088	2	NC	1
593	12	max	.008	3	.24	3	0	1	1.289e-2	3	NC	15	NC	1
594		min	-.005	2	-.324	2	0	15	-2.107e-2	2	270.377	2	NC	1
595	13	max	.008	3	.204	3	0	1	1.032e-2	3	NC	15	NC	1
596		min	-.005	2	-.273	2	0	15	-1.69e-2	2	307.774	2	NC	1
597	14	max	.008	3	.159	3	0	15	7.744e-3	3	NC	15	NC	1
598		min	-.005	2	-.21	2	-.001	1	-1.273e-2	2	371.841	2	NC	1
599	15	max	.007	3	.109	3	0	15	5.17e-3	3	NC	5	NC	1
600		min	-.005	2	-.14	2	-.003	1	-8.557e-3	2	482.406	2	NC	1
601	16	max	.007	3	.056	3	0	15	2.596e-3	3	NC	5	NC	1
602		min	-.005	2	-.071	2	-.005	1	-4.387e-3	2	687.78	2	NC	1
603	17	max	.007	3	.006	3	0	15	2.282e-5	3	NC	5	NC	1
604		min	-.005	2	-.007	2	-.005	1	-3.651e-4	1	1128.085	2	NC	1
605	18	max	.007	3	.046	2	0	15	2.919e-3	3	NC	4	NC	1
606		min	-.005	2	-.039	3	-.004	1	-6.84e-3	2	2401.088	2	NC	1
607	19	max	.007	3	.093	2	0	1	5.943e-3	3	NC	1	NC	1
608		min	-.005	2	-.082	3	0	15	-1.371e-2	2	NC	1	NC	1



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Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



**Anchor Designer™**  
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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

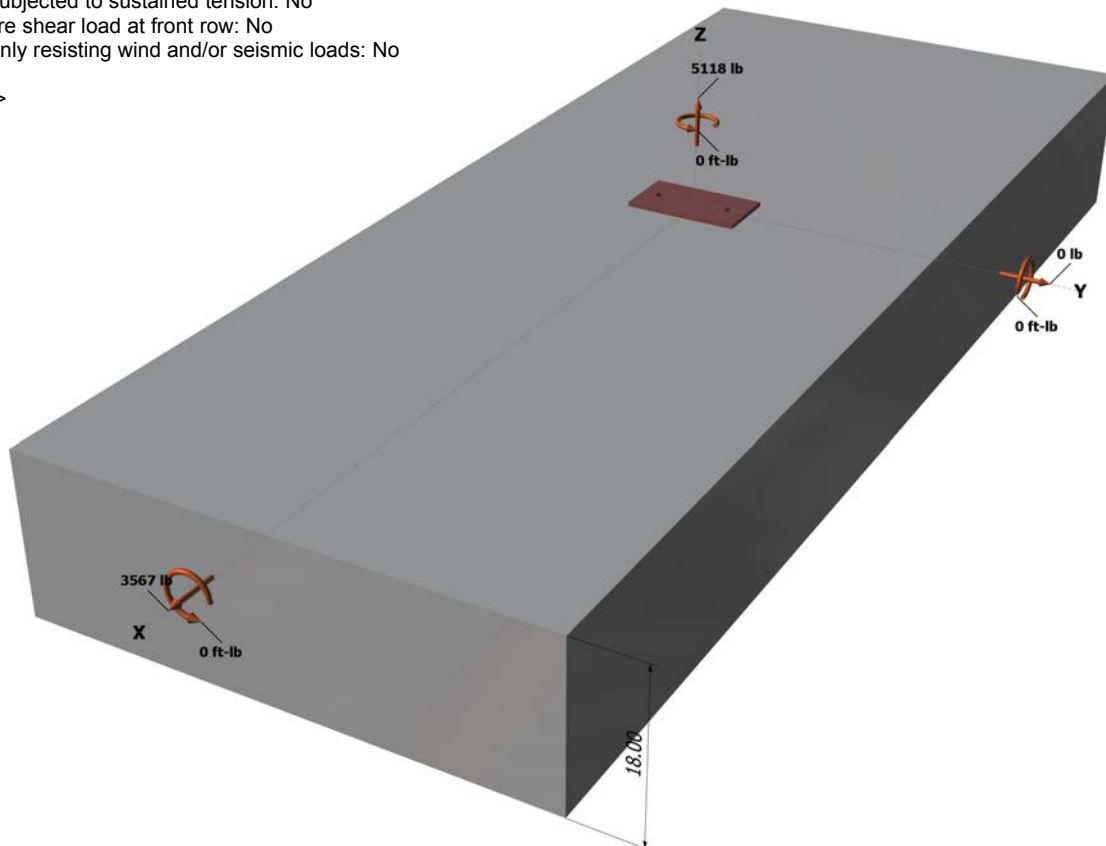
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

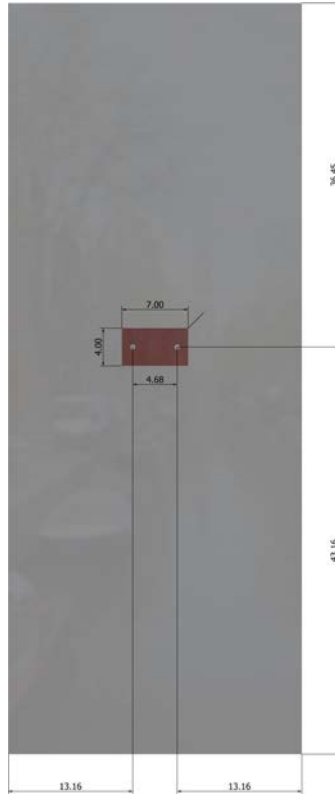
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5118  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
<b>Adhesive</b>	<b>5118</b>	<b>8093</b>	<b>0.63</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1784</b>	<b>3156</b>	<b>0.57</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.