



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	81 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.476 k-ft
M_z =	0.088 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	61%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.188 k-ft
M_z =	0.000 k-ft
P_n =	-1.010 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	95%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	2.393 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.012 k-ft
M_z =	0.000 k-ft
P_n =	2.686 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	46%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.012 k-ft
M_z =	0.000 k-ft
P_n =	3.182 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	37%



5. FOUNDATION DESIGN CALCULATIONS

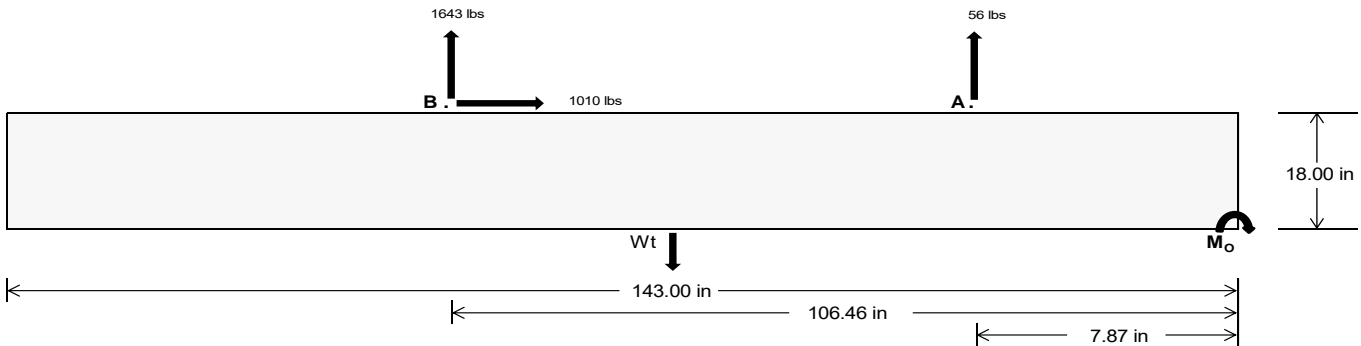
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	242.93	6839.24	k
Compressive Load =	3111.44	4975.94	k
Lateral Load =	10.25	4200.44	k
Moment (Weak Axis) =	0.02	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 193489.5$ in-lbs
Resisting Force Required = 2706.15 lbs
S.F. = 1.67
Weight Required = 4510.24 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 1010.10 lbs
Friction = 0.4
Weight Required = 2525.25 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 1010.10 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$ 7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	948 lbs	948 lbs	948 lbs	948 lbs	1314 lbs	1314 lbs	1314 lbs	1314 lbs	1595 lbs	1595 lbs	1595 lbs	1595 lbs	-112 lbs	-112 lbs	-112 lbs	-112 lbs
F_B	918 lbs	918 lbs	918 lbs	918 lbs	2214 lbs	2214 lbs	2214 lbs	2214 lbs	2251 lbs	2251 lbs	2251 lbs	2251 lbs	-3285 lbs	-3285 lbs	-3285 lbs	-3285 lbs
F_V	112 lbs	112 lbs	112 lbs	112 lbs	1809 lbs	1809 lbs	1809 lbs	1809 lbs	1430 lbs	1430 lbs	1430 lbs	1430 lbs	-2020 lbs	-2020 lbs	-2020 lbs	-2020 lbs
P_{total}	9426 lbs	9642 lbs	9858 lbs	10074 lbs	11089 lbs	11305 lbs	11521 lbs	11737 lbs	11406 lbs	11622 lbs	11838 lbs	12054 lbs	1139 lbs	1268 lbs	1398 lbs	1527 lbs
M	2520 lbs-ft	2520 lbs-ft	2520 lbs-ft	2520 lbs-ft	3233 lbs-ft	3233 lbs-ft	3233 lbs-ft	3233 lbs-ft	4043 lbs-ft	4043 lbs-ft	4043 lbs-ft	4043 lbs-ft	5946 lbs-ft	5946 lbs-ft	5946 lbs-ft	5946 lbs-ft
e	0.27 ft	0.26 ft	0.26 ft	0.25 ft	0.29 ft	0.29 ft	0.28 ft	0.28 ft	0.35 ft	0.35 ft	0.34 ft	0.34 ft	5.22 ft	4.69 ft	4.25 ft	3.89 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	234.7 psf	234.2 psf	233.8 psf	233.3 psf	272.2 psf	270.7 psf	269.2 psf	267.9 psf	269.6 psf	268.1 psf	266.8 psf	265.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	307.7 psf	305.2 psf	302.8 psf	300.6 psf	365.9 psf	361.7 psf	357.8 psf	354.1 psf	386.7 psf	382.0 psf	377.6 psf	373.4 psf	353.6 psf	222.0 psf	177.3 psf	155.7 psf

Maximum Bearing Pressure = 387 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

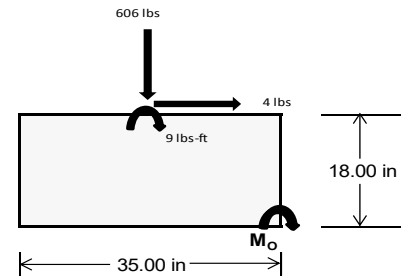
Overturning Check

$M_o = 868.6 \text{ ft-lbs}$
 Resisting Force Required = 595.63 lbs
 S.F. = 1.67
 Weight Required = 992.72 lbs
 Minimum Width = 35 in
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	217 lbs	497 lbs	217 lbs	606 lbs	1572 lbs	606 lbs	63 lbs	145 lbs	63 lbs
F_v	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
P_{total}	9576 lbs	7560 lbs	9576 lbs	9515 lbs	7560 lbs	9515 lbs	2800 lbs	7560 lbs	2800 lbs
M	4 lbs-ft	0 lbs-ft	4 lbs-ft	15 lbs-ft	0 lbs-ft	15 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	275.3 psf	217.5 psf	275.3 psf	272.9 psf	217.5 psf	272.9 psf	80.5 psf	217.5 psf	80.5 psf
f_{max}	275.7 psf	217.5 psf	275.7 psf	274.7 psf	217.5 psf	274.7 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 276 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

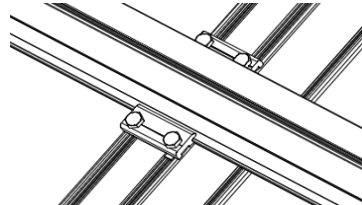
Fastening of Modules to Purlins

Maximum Uplifting Force =	1.045 k
Allowable Uplift =	1.214 k
Utilization =	<u>86%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.528 k
Allowable Uplift =	4.357 k
Utilization =	<u>58%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	2.393 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>32%</u>

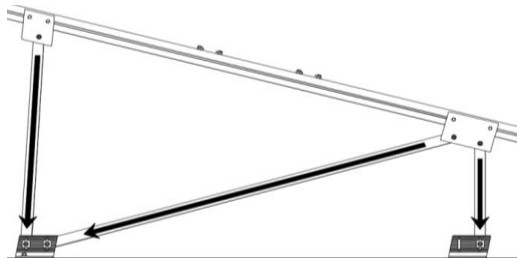
Rear Strut

Maximum Axial Load =	4.607 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>62%</u>

Diagonal Strut

Maximum Axial Load =	2.836 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

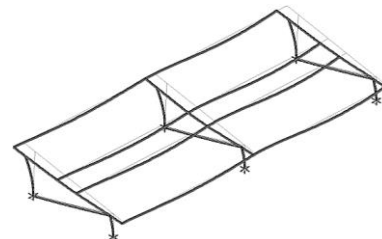
7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	60.93 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.219 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 81 \text{ in}$$

$$J = 0.432$$

$$224.084$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 81$$

$$J = 0.432$$

$$142.504$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

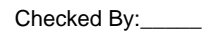
3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 120mph 30psf 6.75ft 7-05PM Page 3d







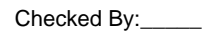
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-32.406	1	-102.112	3	-46.204	1	-.013	3	-.062	1	-.978	2
85		5	max	-1.571	15	-.26	15	.266	10	.017	2	-.003	12	.588	3
86			min	-32.406	1	-37.06	2	-12.979	1	-.013	3	-.084	1	-1.021	2
87		6	max	-1.571	15	85.584	3	20.247	1	.017	2	-.004	15	.559	3
88			min	-32.406	1	-225.799	2	-3.017	3	-.013	3	-.082	1	-.923	2
89		7	max	-1.571	15	179.431	3	53.473	1	.017	2	-.003	15	.46	3
90			min	-32.406	1	-414.539	2	-.669	3	-.013	3	-.054	1	-.683	2
91		8	max	-1.571	15	273.279	3	86.699	1	.017	2	.005	2	.29	3
92			min	-32.406	1	-603.278	2	1.295	12	-.013	3	-.009	3	-.301	2
93		9	max	-1.571	15	367.127	3	119.925	1	.017	2	.076	1	.222	2
94			min	-32.406	1	-792.017	2	2.86	12	-.013	3	-.007	3	.002	15
95		10	max	-1.571	15	460.974	3	153.15	1	.013	3	.178	1	.887	2
96			min	-32.406	1	-980.756	2	4.426	12	-.017	2	-.003	3	-.261	3
97		11	max	-1.571	15	792.017	2	-2.86	12	.013	3	.076	1	.222	2
98			min	-32.406	1	-367.127	3	-119.925	1	-.017	2	-.007	3	.002	15
99		12	max	-1.571	15	603.278	2	-1.295	12	.013	3	.005	2	.29	3
100			min	-32.406	1	-273.279	3	-86.699	1	-.017	2	-.009	3	-.301	2
101		13	max	-1.571	15	414.539	2	.669	3	.013	3	-.003	15	.46	3
102			min	-32.406	1	-179.431	3	-53.473	1	-.017	2	-.054	1	-.683	2
103		14	max	-1.571	15	225.799	2	3.017	3	.013	3	-.004	15	.559	3
104			min	-32.406	1	-85.584	3	-20.247	1	-.017	2	-.082	1	-.923	2
105		15	max	-1.571	15	37.06	2	12.979	1	.013	3	-.003	12	.588	3
106			min	-32.406	1	.26	15	-.266	10	-.017	2	-.084	1	-1.021	2
107		16	max	-1.571	15	102.112	3	46.204	1	.013	3	0	3	.546	3
108			min	-32.406	1	-151.679	2	2.222	15	-.017	2	-.062	1	-.978	2
109		17	max	-1.571	15	195.959	3	79.43	1	.013	3	.007	3	.435	3
110			min	-32.406	1	-340.418	2	3.762	15	-.017	2	-.015	1	-.794	2
111		18	max	-1.571	15	289.807	3	112.656	1	.013	3	.057	1	.253	3
112			min	-32.406	1	-529.157	2	5.303	15	-.017	2	.002	10	-.468	2
113		19	max	-1.571	15	383.655	3	145.882	1	.013	3	.154	1	0	2
114			min	-32.406	1	-717.897	2	6.843	15	-.017	2	.007	15	0	3
115	M16	1	max	-2.437	15	630.666	2	-6.547	15	.005	1	.127	1	0	2
116			min	-50.655	1	-303.089	3	-139.865	1	-.012	3	.006	15	0	3
117		2	max	-2.437	15	441.926	2	-5.007	15	.005	1	.034	1	.192	3
118			min	-50.655	1	-209.242	3	-106.639	1	-.012	3	0	10	-.402	2
119		3	max	-2.437	15	253.187	2	-3.467	15	.005	1	.003	3	.314	3
120			min	-50.655	1	-115.394	3	-73.413	1	-.012	3	-.033	1	-.663	2
121		4	max	-2.437	15	64.448	2	-1.926	15	.005	1	-.002	12	.365	3
122			min	-50.655	1	-21.546	3	-40.187	1	-.012	3	-.076	1	-.782	2
123		5	max	-2.437	15	72.302	3	.629	10	.005	1	-.004	12	.346	3
124			min	-50.655	1	-124.291	2	-6.961	1	-.012	3	-.094	1	-.76	2
125		6	max	-2.437	15	166.149	3	26.265	1	.005	1	-.004	15	.257	3
126			min	-50.655	1	-313.03	2	-1.489	3	-.012	3	-.086	1	-.596	2
127		7	max	-2.437	15	259.997	3	59.49	1	.005	1	-.003	15	.097	3
128			min	-50.655	1	-501.77	2	.704	12	-.012	3	-.054	1	-.29	2
129		8	max	-2.437	15	353.845	3	92.716	1	.005	1	.006	2	.157	2
130			min	-50.655	1	-690.509	2	2.27	12	-.012	3	-.007	3	-.133	3
131		9	max	-2.437	15	447.692	3	125.942	1	.005	1	.085	1	.746	2
132			min	-50.655	1	-879.248	2	3.835	12	-.012	3	-.004	3	-.434	3
133		10	max	-2.437	15	541.54	3	159.168	1	.012	3	.192	1	1.476	2
134			min	-50.655	1	-1067.987	2	5.401	12	-.005	1	.001	12	-.805	3
135		11	max	-2.437	15	879.248	2	-3.835	12	.012	3	.085	1	.746	2
136			min	-50.655	1	-447.692	3	-125.942	1	-.005	1	-.004	3	-.434	3
137		12	max	-2.437	15	690.509	2	-2.27	12	.012	3	.006	2	.157	2
138			min	-50.655	1	-353.845	3	-92.716	1	-.005	1	-.007	3	-.133	3
139		13	max	-2.437	15	501.77	2	-.704	12	.012	3	-.003	15	.097	3
140			min	-50.655	1	-259.997	3	-59.49	1	-.005	1	-.054	1	-.29	2





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-979.684	3	1.474	15	.01	15	0	1	0	15	-.003	3
199		5	max	850.884	2	5.401	4	.211	1	0	5	0	1	-.001	15
200			min	-979.812	3	1.27	15	.01	15	0	1	0	15	-.004	4
201		6	max	850.714	2	4.532	4	.211	1	0	5	0	1	-.002	15
202			min	-979.94	3	1.065	15	.01	15	0	1	0	15	-.007	4
203		7	max	850.543	2	3.663	4	.211	1	0	5	0	1	-.002	15
204			min	-980.068	3	.861	15	.01	15	0	1	0	15	-.009	4
205		8	max	850.373	2	2.794	4	.211	1	0	5	0	1	-.002	15
206			min	-980.195	3	.657	15	.01	15	0	1	0	15	-.01	4
207		9	max	850.203	2	1.925	4	.211	1	0	5	0	1	-.003	15
208			min	-980.323	3	.453	15	.01	15	0	1	0	15	-.011	4
209		10	max	850.032	2	1.056	4	.211	1	0	5	.001	1	-.003	15
210			min	-980.451	3	.248	15	.01	15	0	1	0	15	-.012	4
211		11	max	849.862	2	.319	2	.211	1	0	5	.001	1	-.003	15
212			min	-980.579	3	-.128	3	.01	15	0	1	0	15	-.012	4
213		12	max	849.692	2	-.16	15	.211	1	0	5	.001	1	-.003	15
214			min	-980.707	3	-.682	4	.01	15	0	1	0	15	-.012	4
215		13	max	849.521	2	-.364	15	.211	1	0	5	.001	1	-.003	15
216			min	-980.834	3	-1.551	4	.01	15	0	1	0	15	-.012	4
217		14	max	849.351	2	-.569	15	.211	1	0	5	.001	1	-.003	15
218			min	-980.962	3	-2.419	4	.01	15	0	1	0	15	-.011	4
219		15	max	849.18	2	-.773	15	.211	1	0	5	.002	1	-.002	15
220			min	-981.09	3	-3.288	4	.01	15	0	1	0	15	-.009	4
221		16	max	849.01	2	-.977	15	.211	1	0	5	.002	1	-.002	15
222			min	-981.218	3	-4.157	4	.01	15	0	1	0	15	-.008	4
223		17	max	848.84	2	-1.181	15	.211	1	0	5	.002	1	-.001	15
224			min	-981.345	3	-5.026	4	.01	15	0	1	0	15	-.006	4
225		18	max	848.669	2	-1.386	15	.211	1	0	5	.002	1	0	15
226			min	-981.473	3	-5.895	4	.01	15	0	1	0	15	-.003	4
227		19	max	848.499	2	-1.59	15	.211	1	0	5	.002	1	0	1
228			min	-981.601	3	-6.764	4	.01	15	0	1	0	15	0	1
229	M4	1	max	894.35	1	0	1	-.387	15	0	1	.002	1	0	1
230			min	-29.243	3	0	1	-8.082	1	0	1	0	15	0	1
231		2	max	894.52	1	0	1	-.387	15	0	1	0	1	0	1
232			min	-29.116	3	0	1	-8.082	1	0	1	0	15	0	1
233		3	max	894.69	1	0	1	-.387	15	0	1	0	15	0	1
234			min	-28.988	3	0	1	-8.082	1	0	1	0	1	0	1
235		4	max	894.861	1	0	1	-.387	15	0	1	0	15	0	1
236			min	-28.86	3	0	1	-8.082	1	0	1	-.001	1	0	1
237		5	max	895.031	1	0	1	-.387	15	0	1	0	15	0	1
238			min	-28.732	3	0	1	-8.082	1	0	1	-.002	1	0	1
239		6	max	895.201	1	0	1	-.387	15	0	1	0	15	0	1
240			min	-28.605	3	0	1	-8.082	1	0	1	-.003	1	0	1
241		7	max	895.372	1	0	1	-.387	15	0	1	0	15	0	1
242			min	-28.477	3	0	1	-8.082	1	0	1	-.004	1	0	1
243		8	max	895.542	1	0	1	-.387	15	0	1	0	15	0	1
244			min	-28.349	3	0	1	-8.082	1	0	1	-.005	1	0	1
245		9	max	895.712	1	0	1	-.387	15	0	1	0	15	0	1
246			min	-28.221	3	0	1	-8.082	1	0	1	-.006	1	0	1
247		10	max	895.883	1	0	1	-.387	15	0	1	0	15	0	1
248			min	-28.094	3	0	1	-8.082	1	0	1	-.007	1	0	1
249		11	max	896.053	1	0	1	-.387	15	0	1	0	15	0	1
250			min	-27.966	3	0	1	-8.082	1	0	1	-.008	1	0	1
251		12	max	896.224	1	0	1	-.387	15	0	1	0	15	0	1
252			min	-27.838	3	0	1	-8.082	1	0	1	-.009	1	0	1
253		13	max	896.394	1	0	1	-.387	15	0	1	0	15	0	1
254			min	-27.71	3	0	1	-8.082	1	0	1	-.01	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	896.564	1	0	1	-387	15	0	1	0	15	0	1
256		min	-27.583	3	0	1	-8.082	1	0	1	-.011	1	0	1
257	15	max	896.735	1	0	1	-387	15	0	1	0	15	0	1
258		min	-27.455	3	0	1	-8.082	1	0	1	-.011	1	0	1
259	16	max	896.905	1	0	1	-387	15	0	1	0	15	0	1
260		min	-27.327	3	0	1	-8.082	1	0	1	-.012	1	0	1
261	17	max	897.075	1	0	1	-387	15	0	1	0	15	0	1
262		min	-27.199	3	0	1	-8.082	1	0	1	-.013	1	0	1
263	18	max	897.246	1	0	1	-387	15	0	1	0	15	0	1
264		min	-27.072	3	0	1	-8.082	1	0	1	-.014	1	0	1
265	19	max	897.416	1	0	1	-387	15	0	1	0	15	0	1
266		min	-26.944	3	0	1	-8.082	1	0	1	-.015	1	0	1
267	M6	1	max	3172.66	2	2.281	2	0	1	0	0	1	0	1
268		min	-4606.578	3	.216	12	0	1	0	1	0	1	0	1
269	2	max	3173.189	2	2.226	2	0	1	0	1	0	1	0	12
270		min	-4606.181	3	.189	12	0	1	0	1	0	1	0	2
271	3	max	3173.718	2	2.171	2	0	1	0	1	0	1	0	12
272		min	-4605.784	3	.161	12	0	1	0	1	0	1	-.002	2
273	4	max	3174.248	2	2.115	2	0	1	0	1	0	1	0	12
274		min	-4605.387	3	.133	12	0	1	0	1	0	1	-.002	2
275	5	max	3174.777	2	2.06	2	0	1	0	1	0	1	0	12
276		min	-4604.99	3	.102	3	0	1	0	1	0	1	-.003	2
277	6	max	3175.306	2	2.005	2	0	1	0	1	0	1	0	12
278		min	-4604.593	3	.06	3	0	1	0	1	0	1	-.004	2
279	7	max	3175.836	2	1.949	2	0	1	0	1	0	1	0	12
280		min	-4604.196	3	.019	3	0	1	0	1	0	1	-.005	2
281	8	max	3176.365	2	1.894	2	0	1	0	1	0	1	0	12
282		min	-4603.799	3	-.023	3	0	1	0	1	0	1	-.005	2
283	9	max	3176.894	2	1.839	2	0	1	0	1	0	1	0	3
284		min	-4603.402	3	-.064	3	0	1	0	1	0	1	-.006	2
285	10	max	3177.423	2	1.783	2	0	1	0	1	0	1	0	3
286		min	-4603.005	3	-.106	3	0	1	0	1	0	1	-.007	2
287	11	max	3177.953	2	1.728	2	0	1	0	1	0	1	0	3
288		min	-4602.608	3	-.147	3	0	1	0	1	0	1	-.007	2
289	12	max	3178.482	2	1.673	2	0	1	0	1	0	1	0	3
290		min	-4602.211	3	-.189	3	0	1	0	1	0	1	-.008	2
291	13	max	3179.011	2	1.617	2	0	1	0	1	0	1	0	3
292		min	-4601.814	3	-.23	3	0	1	0	1	0	1	-.008	2
293	14	max	3179.541	2	1.562	2	0	1	0	1	0	1	0	3
294		min	-4601.417	3	-.272	3	0	1	0	1	0	1	-.009	2
295	15	max	3180.07	2	1.507	2	0	1	0	1	0	1	0	3
296		min	-4601.02	3	-.313	3	0	1	0	1	0	1	-.01	2
297	16	max	3180.599	2	1.451	2	0	1	0	1	0	1	0	3
298		min	-4600.623	3	-.355	3	0	1	0	1	0	1	-.01	2
299	17	max	3181.128	2	1.396	2	0	1	0	1	0	1	0	3
300		min	-4600.226	3	-.396	3	0	1	0	1	0	1	-.011	2
301	18	max	3181.658	2	1.34	2	0	1	0	1	0	1	0	3
302		min	-4599.829	3	-.438	3	0	1	0	1	0	1	-.011	2
303	19	max	3182.187	2	1.285	2	0	1	0	1	0	1	0	3
304		min	-4599.432	3	-.479	3	0	1	0	1	0	1	-.012	2
305	M7	1	max	2686.312	2	8.896	4	0	1	0	0	1	.012	2
306		min	-2833.601	3	2.09	15	0	1	0	1	0	1	0	3
307	2	max	2686.141	2	8.027	4	0	1	0	1	0	1	.008	2
308		min	-2833.729	3	1.885	15	0	1	0	1	0	1	-.003	3
309	3	max	2685.971	2	7.158	4	0	1	0	1	0	1	.005	2
310		min	-2833.856	3	1.681	15	0	1	0	1	0	1	-.004	3
311	4	max	2685.801	2	6.289	4	0	1	0	1	0	1	.002	2



Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2392.564	2	0	1	0	1	0	1	0	1	0	1
370			min	-187.51	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2392.734	2	0	1	0	1	0	1	0	1	0	1
372			min	-187.382	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2392.904	2	0	1	0	1	0	1	0	1	0	1
374			min	-187.254	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2393.075	2	0	1	0	1	0	1	0	1	0	1
376			min	-187.126	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2393.245	2	0	1	0	1	0	1	0	1	0	1
378			min	-186.999	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2393.415	2	0	1	0	1	0	1	0	1	0	1
380			min	-186.871	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1078.184	2	2.025	4	-.012	15	0	1	0	2	0	1
382			min	-1516.599	3	.476	15	-.245	1	0	3	0	3	0	1
383		2	max	1078.713	2	1.954	4	-.012	15	0	1	0	10	0	15
384			min	-1516.202	3	.459	15	-.245	1	0	3	0	1	0	4
385		3	max	1079.242	2	1.883	4	-.012	15	0	1	0	15	0	15
386			min	-1515.805	3	.443	15	-.245	1	0	3	0	1	-.001	4
387		4	max	1079.772	2	1.812	4	-.012	15	0	1	0	15	0	15
388			min	-1515.408	3	.426	15	-.245	1	0	3	0	1	-.002	4
389		5	max	1080.301	2	1.741	4	-.012	15	0	1	0	15	0	15
390			min	-1515.011	3	.409	15	-.245	1	0	3	0	1	-.003	4
391		6	max	1080.83	2	1.67	4	-.012	15	0	1	0	15	0	15
392			min	-1514.614	3	.393	15	-.245	1	0	3	0	1	-.003	4
393		7	max	1081.36	2	1.599	4	-.012	15	0	1	0	15	0	15
394			min	-1514.217	3	.376	15	-.245	1	0	3	0	1	-.004	4
395		8	max	1081.889	2	1.528	4	-.012	15	0	1	0	15	-.001	15
396			min	-1513.82	3	.359	15	-.245	1	0	3	0	1	-.004	4
397		9	max	1082.418	2	1.457	4	-.012	15	0	1	0	15	-.001	15
398			min	-1513.423	3	.342	15	-.245	1	0	3	0	1	-.005	4
399		10	max	1082.947	2	1.386	4	-.012	15	0	1	0	15	-.001	15
400			min	-1513.026	3	.326	15	-.245	1	0	3	0	1	-.006	4
401		11	max	1083.477	2	1.315	4	-.012	15	0	1	0	15	-.001	15
402			min	-1512.629	3	.309	15	-.245	1	0	3	0	1	-.006	4
403		12	max	1084.006	2	1.243	4	-.012	15	0	1	0	15	-.002	15
404			min	-1512.232	3	.29	12	-.245	1	0	3	0	1	-.006	4
405		13	max	1084.535	2	1.172	4	-.012	15	0	1	0	15	-.002	15
406			min	-1511.835	3	.263	12	-.245	1	0	3	-.001	1	-.007	4
407		14	max	1085.065	2	1.101	4	-.012	15	0	1	0	15	-.002	15
408			min	-1511.438	3	.235	12	-.245	1	0	3	-.001	1	-.007	4
409		15	max	1085.594	2	1.03	4	-.012	15	0	1	0	15	-.002	15
410			min	-1511.041	3	.207	12	-.245	1	0	3	-.001	1	-.008	4
411		16	max	1086.123	2	.975	2	-.012	15	0	1	0	15	-.002	15
412			min	-1510.644	3	.18	12	-.245	1	0	3	-.001	1	-.008	4
413		17	max	1086.653	2	.92	2	-.012	15	0	1	0	15	-.002	15
414			min	-1510.247	3	.152	12	-.245	1	0	3	-.001	1	-.008	4
415		18	max	1087.182	2	.864	2	-.012	15	0	1	0	15	-.002	15
416			min	-1509.85	3	.124	12	-.245	1	0	3	-.001	1	-.009	4
417		19	max	1087.711	2	.809	2	-.012	15	0	1	0	15	-.002	15
418			min	-1509.453	3	.097	12	-.245	1	0	3	-.002	1	-.009	4
419	M11	1	max	851.565	2	8.876	4	-.01	15	0	1	0	15	.009	4
420			min	-979.301	3	2.087	15	-.211	1	0	5	0	1	.002	15
421		2	max	851.395	2	8.007	4	-.01	15	0	1	0	15	.005	2
422			min	-979.429	3	1.882	15	-.211	1	0	5	0	1	0	12
423		3	max	851.225	2	7.138	4	-.01	15	0	1	0	15	.002	2
424			min	-979.557	3	1.678	15	-.211	1	0	5	0	1	0	3
425		4	max	851.054	2	6.269	4	-.01	15	0	1	0	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-979.684	3	1.474	15	-.211	1	0	5	0	1	-.003	3
427		5	max	850.884	2	5.401	4	-.01	15	0	1	0	15	-.001	15
428			min	-979.812	3	1.27	15	-.211	1	0	5	0	1	-.004	4
429		6	max	850.714	2	4.532	4	-.01	15	0	1	0	15	-.002	15
430			min	-979.94	3	1.065	15	-.211	1	0	5	0	1	-.007	4
431		7	max	850.543	2	3.663	4	-.01	15	0	1	0	15	-.002	15
432			min	-980.068	3	.861	15	-.211	1	0	5	0	1	-.009	4
433		8	max	850.373	2	2.794	4	-.01	15	0	1	0	15	-.002	15
434			min	-980.195	3	.657	15	-.211	1	0	5	0	1	-.01	4
435		9	max	850.203	2	1.925	4	-.01	15	0	1	0	15	-.003	15
436			min	-980.323	3	.453	15	-.211	1	0	5	0	1	-.011	4
437		10	max	850.032	2	1.056	4	-.01	15	0	1	0	15	-.003	15
438			min	-980.451	3	.248	15	-.211	1	0	5	-.001	1	-.012	4
439		11	max	849.862	2	.319	2	-.01	15	0	1	0	15	-.003	15
440			min	-980.579	3	-.128	3	-.211	1	0	5	-.001	1	-.012	4
441		12	max	849.692	2	-.16	15	-.01	15	0	1	0	15	-.003	15
442			min	-980.707	3	-.682	4	-.211	1	0	5	-.001	1	-.012	4
443		13	max	849.521	2	-.364	15	-.01	15	0	1	0	15	-.003	15
444			min	-980.834	3	-1.551	4	-.211	1	0	5	-.001	1	-.012	4
445		14	max	849.351	2	-.569	15	-.01	15	0	1	0	15	-.003	15
446			min	-980.962	3	-2.419	4	-.211	1	0	5	-.001	1	-.011	4
447		15	max	849.18	2	-.773	15	-.01	15	0	1	0	15	-.002	15
448			min	-981.09	3	-3.288	4	-.211	1	0	5	-.002	1	-.009	4
449		16	max	849.01	2	-.977	15	-.01	15	0	1	0	15	-.002	15
450			min	-981.218	3	-4.157	4	-.211	1	0	5	-.002	1	-.008	4
451		17	max	848.84	2	-1.181	15	-.01	15	0	1	0	15	-.001	15
452			min	-981.345	3	-5.026	4	-.211	1	0	5	-.002	1	-.006	4
453		18	max	848.669	2	-1.386	15	-.01	15	0	1	0	15	0	15
454			min	-981.473	3	-5.895	4	-.211	1	0	5	-.002	1	-.003	4
455		19	max	848.499	2	-1.59	15	-.01	15	0	1	0	15	0	1
456			min	-981.601	3	-6.764	4	-.211	1	0	5	-.002	1	0	1
457	M12	1	max	894.35	1	0	1	8.082	1	0	1	0	15	0	1
458			min	-29.243	3	0	1	.387	15	0	1	-.002	1	0	1
459		2	max	894.52	1	0	1	8.082	1	0	1	0	15	0	1
460			min	-29.116	3	0	1	.387	15	0	1	0	1	0	1
461		3	max	894.69	1	0	1	8.082	1	0	1	0	1	0	1
462			min	-28.988	3	0	1	.387	15	0	1	0	15	0	1
463		4	max	894.861	1	0	1	8.082	1	0	1	.001	1	0	1
464			min	-28.86	3	0	1	.387	15	0	1	0	15	0	1
465		5	max	895.031	1	0	1	8.082	1	0	1	.002	1	0	1
466			min	-28.732	3	0	1	.387	15	0	1	0	15	0	1
467		6	max	895.201	1	0	1	8.082	1	0	1	.003	1	0	1
468			min	-28.605	3	0	1	.387	15	0	1	0	15	0	1
469		7	max	895.372	1	0	1	8.082	1	0	1	.004	1	0	1
470			min	-28.477	3	0	1	.387	15	0	1	0	15	0	1
471		8	max	895.542	1	0	1	8.082	1	0	1	.005	1	0	1
472			min	-28.349	3	0	1	.387	15	0	1	0	15	0	1
473		9	max	895.712	1	0	1	8.082	1	0	1	.006	1	0	1
474			min	-28.221	3	0	1	.387	15	0	1	0	15	0	1
475		10	max	895.883	1	0	1	8.082	1	0	1	.007	1	0	1
476			min	-28.094	3	0	1	.387	15	0	1	0	15	0	1
477		11	max	896.053	1	0	1	8.082	1	0	1	.008	1	0	1
478			min	-27.966	3	0	1	.387	15	0	1	0	15	0	1
479		12	max	896.224	1	0	1	8.082	1	0	1	.009	1	0	1
480			min	-27.838	3	0	1	.387	15	0	1	0	15	0	1
481		13	max	896.394	1	0	1	8.082	1	0	1	.01	1	0	1
482			min	-27.71	3	0	1	.387	15	0	1	0	15	0	1







Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	670.742	3	422.147	3	43.007	1	0	3	0	15	.428	3
598		min	-254.763	2	-669.195	2	2.061	15	0	2	-.014	1	-.483	2
599	15	max	671.374	3	421.053	3	43.007	1	0	3	.012	1	.166	3
600		min	-253.92	2	-670.654	2	2.061	15	0	2	0	15	-.081	1
601	16	max	672.006	3	419.958	3	43.007	1	0	3	.039	1	.35	2
602		min	-253.078	2	-672.113	2	2.061	15	0	2	.002	15	-.095	3
603	17	max	672.638	3	418.864	3	43.007	1	0	3	.066	1	.767	2
604		min	-252.235	2	-673.572	2	2.061	15	0	2	.003	15	-.355	3
605	18	max	-6.802	15	632.905	2	50.71	1	0	2	.095	1	.388	2
606		min	-140.703	1	-302.152	3	2.437	15	0	3	.005	15	-.176	3
607	19	max	-6.547	15	631.446	2	50.71	1	0	2	.127	1	.012	3
608		min	-139.86	1	-303.246	3	2.437	15	0	3	.006	15	-.005	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.224	2	.012	3	1.549e-2	2	NC	1	NC	1
2			min	0	15	-.073	3	-.008	2	-5.08e-3	3	NC	1	NC	1
3		2	max	0	1	.179	2	.014	3	1.627e-2	2	NC	4	NC	1
4			min	0	15	.004	15	-.004	10	-4.548e-3	3	1308.537	3	NC	1
5		3	max	0	1	.153	3	.025	1	1.706e-2	2	NC	4	NC	2
6			min	0	15	.003	15	-.003	10	-4.015e-3	3	716.68	3	6175.913	1
7		4	max	0	1	.219	3	.036	1	1.784e-2	2	NC	4	NC	2
8			min	0	15	.003	15	-.003	10	-3.482e-3	3	554.539	3	4270.905	1
9		5	max	0	1	.242	3	.041	1	1.863e-2	2	NC	4	NC	2
10			min	0	15	.003	15	-.004	10	-2.949e-3	3	514.113	3	3775.312	1
11		6	max	0	1	.222	3	.038	1	1.941e-2	2	NC	4	NC	2
12			min	0	15	.003	15	-.006	10	-2.417e-3	3	548.049	3	4087.849	1
13		7	max	0	1	.211	2	.032	3	2.02e-2	2	NC	2	NC	2
14			min	0	15	.004	15	-.009	10	-1.884e-3	3	669.838	3	5627.981	1
15		8	max	0	1	.262	2	.034	3	2.098e-2	2	NC	4	NC	1
16			min	0	15	.005	15	-.014	2	-1.351e-3	3	954.438	3	7464.171	3
17		9	max	0	1	.307	2	.035	3	2.177e-2	2	NC	4	NC	1
18			min	0	15	.006	15	-.021	2	-8.186e-4	3	1577.106	3	7176.196	3
19	10	max	0	1	.326	2	.035	3	2.255e-2	2	NC	4	NC	1	
20		min	0	1	-.001	3	-.025	2	-2.859e-4	3	1583.001	2	7105.981	3	
21	11	max	0	15	.307	2	.035	3	2.177e-2	2	NC	4	NC	1	
22		min	0	1	.006	15	-.021	2	-8.186e-4	3	1577.106	3	7176.196	3	
23	12	max	0	15	.262	2	.034	3	2.098e-2	2	NC	4	NC	1	
24		min	0	1	.005	15	-.014	2	-1.351e-3	3	954.438	3	7464.171	3	
25	13	max	0	15	.211	2	.032	3	2.02e-2	2	NC	2	NC	2	
26		min	0	1	.004	15	-.009	10	-1.884e-3	3	669.838	3	5627.981	1	
27	14	max	0	15	.222	3	.038	1	1.941e-2	2	NC	4	NC	2	
28		min	0	1	.003	15	-.006	10	-2.417e-3	3	548.049	3	4087.849	1	
29	15	max	0	15	.242	3	.041	1	1.863e-2	2	NC	4	NC	2	
30		min	0	1	.003	15	-.004	10	-2.949e-3	3	514.113	3	3775.312	1	
31	16	max	0	15	.219	3	.036	1	1.784e-2	2	NC	4	NC	2	
32		min	0	1	.003	15	-.003	10	-3.482e-3	3	554.539	3	4270.905	1	
33	17	max	0	15	.153	3	.025	1	1.706e-2	2	NC	4	NC	2	
34		min	0	1	.003	15	-.003	10	-4.015e-3	3	716.68	3	6175.913	1	
35	18	max	0	15	.179	2	.014	3	1.627e-2	2	NC	4	NC	1	
36		min	0	1	.004	15	-.004	10	-4.548e-3	3	1308.537	3	NC	1	
37	19	max	0	15	.224	2	.012	3	1.549e-2	2	NC	1	NC	1	
38		min	0	1	-.073	3	-.008	2	-5.08e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.479	3	.011	3	8.391e-3	2	NC	1	NC	1
40			min	0	15	-.66	2	-.007	2	-7.048e-3	3	NC	1	NC	1





Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMax Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.329	2	-.018	2	-1.758e-2	2	241.527	2	8813.422	3
99		12	max	0	1	1.038	3	.028	3	1.197e-2	3	NC	15	NC	1
100			min	0	15	-1.354	2	-.012	2	-1.648e-2	2	232.75	2	9195.695	3
101		13	max	0	1	1.027	3	.026	3	1.113e-2	3	NC	15	NC	2
102			min	0	15	-1.367	2	-.007	10	-1.537e-2	2	228.68	2	6228.776	1
103		14	max	0	1	.995	3	.034	1	1.028e-2	3	NC	15	NC	2
104			min	0	15	-1.349	2	-.005	10	-1.427e-2	2	234.489	2	4659.599	1
105		15	max	0	1	.939	3	.035	1	9.441e-3	3	NC	5	NC	2
106			min	0	15	-1.291	2	-.003	10	-1.317e-2	2	256.065	2	4453.099	1
107		16	max	0	1	.857	3	.03	1	8.597e-3	3	NC	5	NC	2
108			min	0	15	-1.188	2	-.003	10	-1.206e-2	2	306.016	2	5294.029	1
109		17	max	0	1	.751	3	.018	1	7.753e-3	3	NC	5	NC	2
110			min	0	15	-1.041	2	-.003	10	-1.096e-2	2	423.006	2	8341.249	1
111		18	max	0	1	.625	3	.011	3	6.909e-3	3	NC	5	NC	1
112			min	0	15	-.86	2	-.004	2	-9.855e-3	2	803.33	2	NC	1
113		19	max	0	1	.489	3	.01	3	6.065e-3	3	NC	1	NC	1
114			min	0	15	-.658	2	-.007	2	-8.751e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.199	2	.009	3	1.178e-2	3	NC	1	NC	1
116			min	0	1	-.174	3	-.006	2	-1.303e-2	2	NC	1	NC	1
117		2	max	0	15	.115	2	.01	1	1.257e-2	3	NC	4	NC	1
118			min	0	1	-.147	3	-.003	10	-1.33e-2	2	1935.264	2	NC	1
119		3	max	0	15	.063	1	.025	1	1.336e-2	3	NC	4	NC	2
120			min	0	1	-.128	3	-.002	10	-1.357e-2	2	1080.06	2	6162.606	1
121		4	max	0	15	.038	1	.037	1	1.415e-2	3	NC	4	NC	2
122			min	0	1	-.122	3	-.001	10	-1.385e-2	2	865.333	2	4236.123	1
123		5	max	0	15	.038	1	.042	1	1.494e-2	3	NC	4	NC	2
124			min	0	1	-.133	3	-.002	10	-1.412e-2	2	852.402	2	3718.542	1
125		6	max	0	15	.063	1	.04	1	1.573e-2	3	NC	3	NC	2
126			min	0	1	-.159	3	-.003	10	-1.439e-2	2	1015.59	2	3983.216	1
127		7	max	0	15	.106	1	.029	1	1.652e-2	3	NC	4	NC	2
128			min	0	1	-.196	3	-.006	10	-1.466e-2	2	1574.073	2	5362.939	1
129		8	max	0	15	.165	2	.024	3	1.731e-2	3	NC	1	NC	1
130			min	0	1	-.237	3	-.009	2	-1.493e-2	2	2578.067	3	NC	1
131		9	max	0	15	.227	2	.024	3	1.81e-2	3	NC	4	NC	1
132			min	0	1	-.272	3	-.016	2	-1.52e-2	2	1657.109	3	NC	1
133		10	max	0	1	.254	2	.024	3	1.889e-2	3	NC	4	NC	1
134			min	0	1	-.287	3	-.019	2	-1.548e-2	2	1432.898	3	NC	1
135		11	max	0	1	.227	2	.024	3	1.81e-2	3	NC	4	NC	1
136			min	0	15	-.272	3	-.016	2	-1.52e-2	2	1657.109	3	NC	1
137		12	max	0	1	.165	2	.024	3	1.731e-2	3	NC	1	NC	1
138			min	0	15	-.237	3	-.009	2	-1.493e-2	2	2578.067	3	NC	1
139		13	max	0	1	.106	1	.029	1	1.652e-2	3	NC	4	NC	2
140			min	0	15	-.196	3	-.006	10	-1.466e-2	2	1574.073	2	5362.939	1
141		14	max	0	1	.063	1	.04	1	1.573e-2	3	NC	3	NC	2
142			min	0	15	-.159	3	-.003	10	-1.439e-2	2	1015.59	2	3983.216	1
143		15	max	0	1	.038	1	.042	1	1.494e-2	3	NC	4	NC	2
144			min	0	15	-.133	3	-.002	10	-1.412e-2	2	852.402	2	3718.542	1
145		16	max	0	1	.038	1	.037	1	1.415e-2	3	NC	4	NC	2
146			min	0	15	-.122	3	-.001	10	-1.385e-2	2	865.333	2	4236.123	1
147		17	max	0	1	.063	1	.025	1	1.336e-2	3	NC	4	NC	2
148			min	0	15	-.128	3	-.002	10	-1.357e-2	2	1080.06	2	6162.606	1
149		18	max	0	1	.115	2	.01	1	1.257e-2	3	NC	4	NC	1
150			min	0	15	-.147	3	-.003	10	-1.33e-2	2	1935.264	2	NC	1
151		19	max	0	1	.199	2	.009	3	1.178e-2	3	NC	1	NC	1
152			min	0	15	-.174	3	-.006	2	-1.303e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.012	2	.006	1	-6.135e-6	15	NC	1	NC	1
154			min	-.011	3	-.018	3	0	15	-1.264e-4	1	6424.542	2	NC	1



Company : Schletter, Inc.
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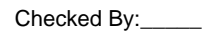
Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.008	2	.01	2	.005	1	-5.836e-6	15	NC	1	NC	1
156			min	-.011	3	-.018	3	0	15	-1.202e-4	1	7447.873	2	NC	1
157		3	max	.007	2	.009	2	.005	1	-5.537e-6	15	NC	1	NC	1
158			min	-.01	3	-.017	3	0	15	-1.141e-4	1	8837.852	2	NC	1
159		4	max	.007	2	.007	2	.004	1	-5.239e-6	15	NC	1	NC	1
160			min	-.009	3	-.017	3	0	15	-1.079e-4	1	NC	1	NC	1
161		5	max	.006	2	.006	2	.004	1	-4.94e-6	15	NC	1	NC	1
162			min	-.009	3	-.016	3	0	15	-1.018e-4	1	NC	1	NC	1
163		6	max	.006	2	.004	2	.003	1	-4.641e-6	15	NC	1	NC	1
164			min	-.008	3	-.015	3	0	15	-9.559e-5	1	NC	1	NC	1
165		7	max	.005	2	.003	2	.003	1	-4.342e-6	15	NC	1	NC	1
166			min	-.008	3	-.014	3	0	15	-8.943e-5	1	NC	1	NC	1
167		8	max	.005	2	.002	2	.003	1	-4.044e-6	15	NC	1	NC	1
168			min	-.007	3	-.014	3	0	15	-8.327e-5	1	NC	1	NC	1
169		9	max	.004	2	0	2	.002	1	-3.745e-6	15	NC	1	NC	1
170			min	-.006	3	-.013	3	0	15	-7.711e-5	1	NC	1	NC	1
171		10	max	.004	2	0	2	.002	1	-3.446e-6	15	NC	1	NC	1
172			min	-.006	3	-.012	3	0	15	-7.095e-5	1	NC	1	NC	1
173		11	max	.004	2	-.001	2	.001	1	-3.148e-6	15	NC	1	NC	1
174			min	-.005	3	-.011	3	0	15	-6.478e-5	1	NC	1	NC	1
175		12	max	.003	2	-.002	15	.001	1	-2.849e-6	15	NC	1	NC	1
176			min	-.004	3	-.01	3	0	15	-5.862e-5	1	NC	1	NC	1
177		13	max	.003	2	-.002	15	0	1	-2.55e-6	15	NC	1	NC	1
178			min	-.004	3	-.009	3	0	15	-5.246e-5	1	NC	1	NC	1
179		14	max	.002	2	-.002	15	0	1	-2.252e-6	15	NC	1	NC	1
180			min	-.003	3	-.008	3	0	15	-4.63e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-1.953e-6	15	NC	1	NC	1
182			min	-.003	3	-.006	3	0	15	-4.014e-5	1	NC	1	NC	1
183		16	max	.001	2	-.001	15	0	1	-1.654e-6	15	NC	1	NC	1
184			min	-.002	3	-.005	3	0	15	-3.398e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-1.356e-6	15	NC	1	NC	1
186			min	-.001	3	-.003	4	0	15	-2.782e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-1.057e-6	15	NC	1	NC	1
188			min	0	3	-.002	4	0	15	-2.166e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-7.582e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.55e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	3.155e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	1.548e-7	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	1.801e-5	1	NC	1	NC	1
194			min	0	2	-.003	4	0	1	8.635e-7	15	NC	1	NC	1
195		3	max	.001	3	-.001	15	0	15	3.286e-5	1	NC	1	NC	1
196			min	0	2	-.006	4	0	1	1.572e-6	15	NC	1	NC	1
197		4	max	.002	3	-.002	15	0	15	4.772e-5	1	NC	1	NC	1
198			min	-.001	2	-.009	4	0	1	2.281e-6	15	NC	1	NC	1
199		5	max	.002	3	-.003	15	0	15	6.257e-5	1	NC	1	NC	1
200			min	-.002	2	-.012	4	0	1	2.99e-6	15	8393.859	4	NC	1
201		6	max	.003	3	-.004	15	0	15	7.743e-5	1	NC	2	NC	1
202			min	-.002	2	-.015	4	0	1	3.698e-6	15	6810.926	4	NC	1
203		7	max	.003	3	-.004	15	0	10	9.228e-5	1	NC	5	NC	1
204			min	-.003	2	-.018	4	0	1	4.407e-6	15	5857.17	4	NC	1
205		8	max	.004	3	-.005	15	0	10	1.071e-4	1	NC	5	NC	1
206			min	-.003	2	-.02	4	0	1	5.116e-6	15	5269.095	4	NC	1
207		9	max	.004	3	-.005	15	0	1	1.22e-4	1	NC	5	NC	1
208			min	-.004	2	-.021	4	0	3	5.825e-6	15	4922.652	4	NC	1
209		10	max	.005	3	-.005	15	0	1	1.368e-4	1	NC	5	NC	1
210			min	-.004	2	-.022	4	0	12	6.533e-6	15	4757.759	4	NC	1
211		11	max	.005	3	-.005	15	0	1	1.517e-4	1	NC	5	NC	1



RISA-3D Version 13.0.0 \.....\PVMMax 72 Cell 2V 30° 120mph 30psf 6.75ft 7-05-18 Page 34







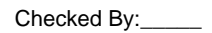
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Dec 1, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.008	2	.01	2	0	15	1.202e-4	1	NC	1	NC	1
384		min	-.011	3	-.018	3	-.005	1	5.836e-6	15	7447.873	2	NC	1
385	3	max	.007	2	.009	2	0	15	1.141e-4	1	NC	1	NC	1
386		min	-.01	3	-.017	3	-.005	1	5.537e-6	15	8837.852	2	NC	1
387	4	max	.007	2	.007	2	0	15	1.079e-4	1	NC	1	NC	1
388		min	-.009	3	-.017	3	-.004	1	5.239e-6	15	NC	1	NC	1
389	5	max	.006	2	.006	2	0	15	1.018e-4	1	NC	1	NC	1
390		min	-.009	3	-.016	3	-.004	1	4.94e-6	15	NC	1	NC	1
391	6	max	.006	2	.004	2	0	15	9.559e-5	1	NC	1	NC	1
392		min	-.008	3	-.015	3	-.003	1	4.641e-6	15	NC	1	NC	1
393	7	max	.005	2	.003	2	0	15	8.943e-5	1	NC	1	NC	1
394		min	-.008	3	-.014	3	-.003	1	4.342e-6	15	NC	1	NC	1
395	8	max	.005	2	.002	2	0	15	8.327e-5	1	NC	1	NC	1
396		min	-.007	3	-.014	3	-.003	1	4.044e-6	15	NC	1	NC	1
397	9	max	.004	2	0	2	0	15	7.711e-5	1	NC	1	NC	1
398		min	-.006	3	-.013	3	-.002	1	3.745e-6	15	NC	1	NC	1
399	10	max	.004	2	0	2	0	15	7.095e-5	1	NC	1	NC	1
400		min	-.006	3	-.012	3	-.002	1	3.446e-6	15	NC	1	NC	1
401	11	max	.004	2	-.001	2	0	15	6.478e-5	1	NC	1	NC	1
402		min	-.005	3	-.011	3	-.001	1	3.148e-6	15	NC	1	NC	1
403	12	max	.003	2	-.002	15	0	15	5.862e-5	1	NC	1	NC	1
404		min	-.004	3	-.01	3	-.001	1	2.849e-6	15	NC	1	NC	1
405	13	max	.003	2	-.002	15	0	15	5.246e-5	1	NC	1	NC	1
406		min	-.004	3	-.009	3	0	1	2.55e-6	15	NC	1	NC	1
407	14	max	.002	2	-.002	15	0	15	4.63e-5	1	NC	1	NC	1
408		min	-.003	3	-.008	3	0	1	2.252e-6	15	NC	1	NC	1
409	15	max	.002	2	-.001	15	0	15	4.014e-5	1	NC	1	NC	1
410		min	-.003	3	-.006	3	0	1	1.953e-6	15	NC	1	NC	1
411	16	max	.001	2	-.001	15	0	15	3.398e-5	1	NC	1	NC	1
412		min	-.002	3	-.005	3	0	1	1.654e-6	15	NC	1	NC	1
413	17	max	0	2	0	15	0	15	2.782e-5	1	NC	1	NC	1
414		min	-.001	3	-.003	4	0	1	1.356e-6	15	NC	1	NC	1
415	18	max	0	2	0	15	0	15	2.166e-5	1	NC	1	NC	1
416		min	0	3	-.002	4	0	1	1.057e-6	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	1.55e-5	1	NC	1	NC	1
418		min	0	1	0	1	0	1	7.582e-7	15	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-1.548e-7	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-3.155e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-8.635e-7	15	NC	1	NC	1
422		min	0	2	-.003	4	0	15	-1.801e-5	1	NC	1	NC	1
423	3	max	.001	3	-.001	15	0	1	-1.572e-6	15	NC	1	NC	1
424		min	0	2	-.006	4	0	15	-3.286e-5	1	NC	1	NC	1
425	4	max	.002	3	-.002	15	0	1	-2.281e-6	15	NC	1	NC	1
426		min	-.001	2	-.009	4	0	15	-4.772e-5	1	NC	1	NC	1
427	5	max	.002	3	-.003	15	0	1	-2.99e-6	15	NC	1	NC	1
428		min	-.002	2	-.012	4	0	15	-6.257e-5	1	8393.859	4	NC	1
429	6	max	.003	3	-.004	15	0	1	-3.698e-6	15	NC	2	NC	1
430		min	-.002	2	-.015	4	0	15	-7.743e-5	1	6810.926	4	NC	1
431	7	max	.003	3	-.004	15	0	1	-4.407e-6	15	NC	5	NC	1
432		min	-.003	2	-.018	4	0	10	-9.228e-5	1	5857.17	4	NC	1
433	8	max	.004	3	-.005	15	0	1	-5.116e-6	15	NC	5	NC	1
434		min	-.003	2	-.02	4	0	10	-1.071e-4	1	5269.095	4	NC	1
435	9	max	.004	3	-.005	15	0	3	-5.825e-6	15	NC	5	NC	1
436		min	-.004	2	-.021	4	0	1	-1.22e-4	1	4922.652	4	NC	1
437	10	max	.005	3	-.005	15	0	12	-6.533e-6	15	NC	5	NC	1
438		min	-.004	2	-.022	4	0	1	-1.368e-4	1	4757.759	4	NC	1
439	11	max	.005	3	-.005	15	0	15	-7.242e-6	15	NC	5	NC	1









Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

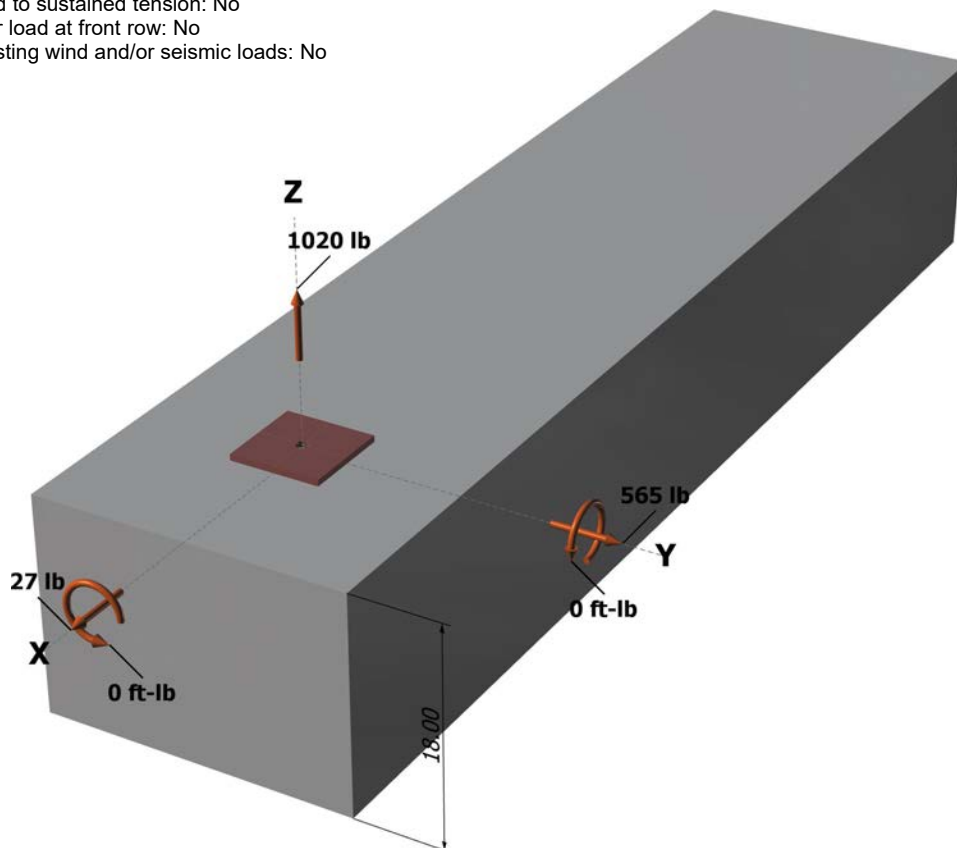
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1020
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cby} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cby} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

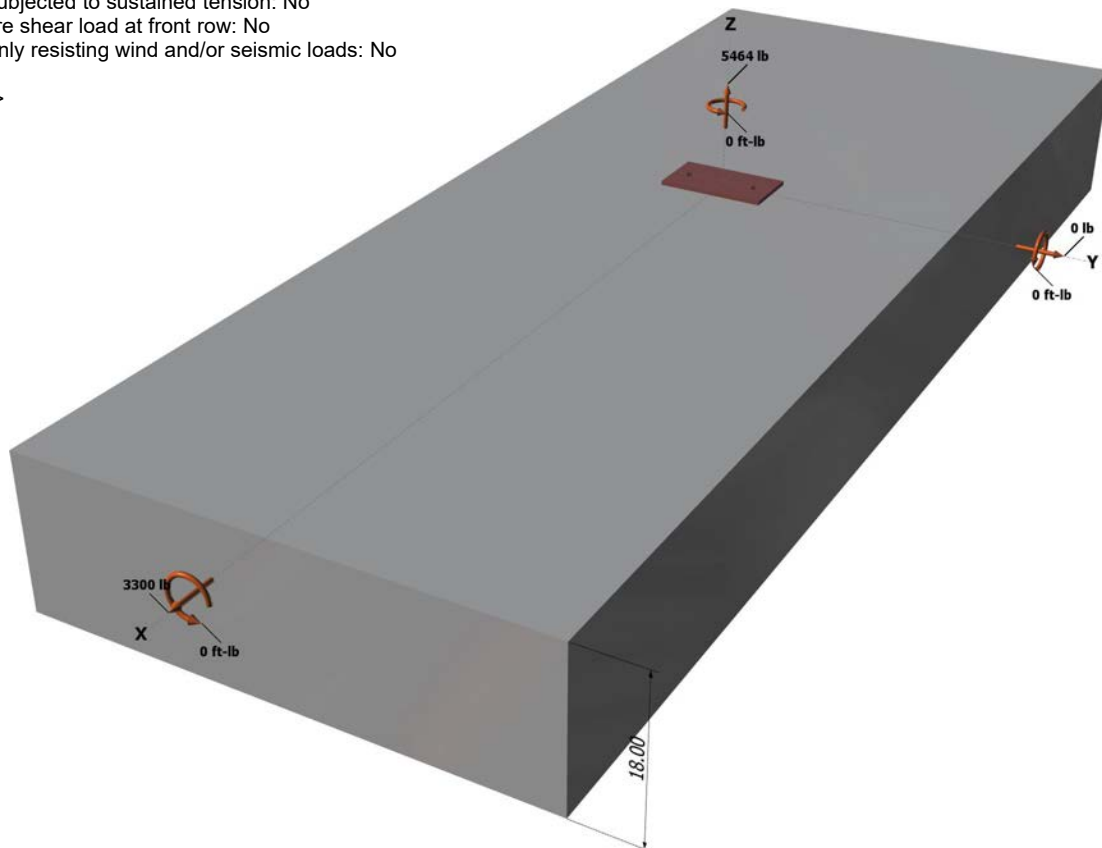
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



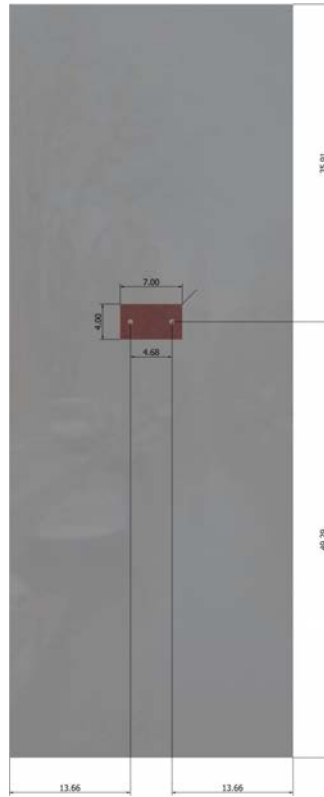
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.