

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

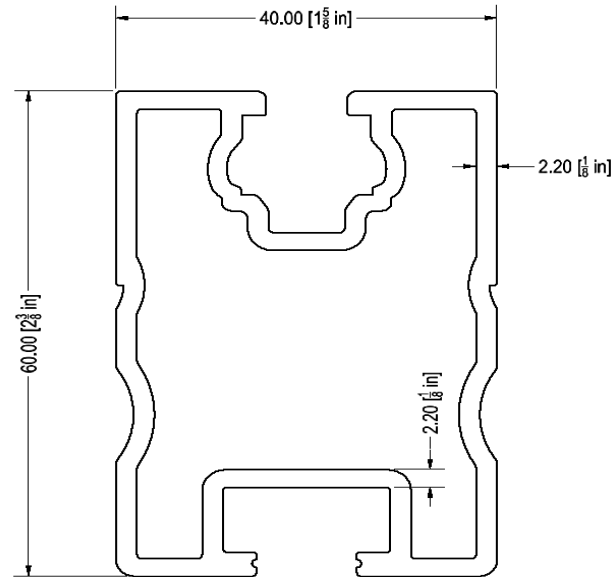
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	54 in
ΦF_{ty} STRONG-AXIS =	29.52 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.453 k-ft
M_z =	0.060 k-ft
$M_{y \text{ allowable}}$ =	1.256 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	43%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.82 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.499 k-ft
M_z =	0.000 k-ft
P_n =	0.229 k
$M_{y \text{ allowable}}$ =	1.463 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	36%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.139 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.190 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	5%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.869 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	11%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.060 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 40 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

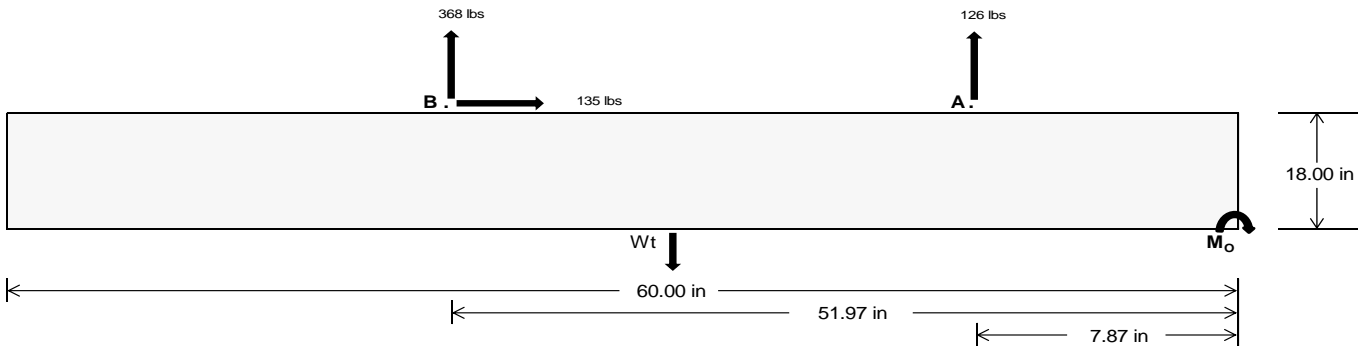
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	526.63	1532.12	k
Compressive Load =	1480.22	1066.86	k
Lateral Load =	1.55	560.58	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 22528.3$ in-lbs
Resisting Force Required = 750.94 lbs
S.F. = 1.67
Weight Required = 1251.57 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 134.76 lbs
Friction = 0.4
Weight Required = 336.89 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 134.76 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	471 lbs	471 lbs	471 lbs	471 lbs	580 lbs	580 lbs	580 lbs	580 lbs	756 lbs	756 lbs	756 lbs	756 lbs	-252 lbs	-252 lbs	-252 lbs	-252 lbs
F_B	341 lbs	341 lbs	341 lbs	341 lbs	417 lbs	417 lbs	417 lbs	417 lbs	544 lbs	544 lbs	544 lbs	544 lbs	-736 lbs	-736 lbs	-736 lbs	-736 lbs
F_V	26 lbs	26 lbs	26 lbs	26 lbs	235 lbs	235 lbs	235 lbs	235 lbs	194 lbs	194 lbs	194 lbs	194 lbs	-270 lbs	-270 lbs	-270 lbs	-270 lbs
P_{total}	2806 lbs	2897 lbs	2988 lbs	3078 lbs	2991 lbs	3081 lbs	3172 lbs	3263 lbs	3293 lbs	3384 lbs	3475 lbs	3565 lbs	209 lbs	264 lbs	318 lbs	372 lbs
M	283 lbs-ft	283 lbs-ft	283 lbs-ft	283 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	689 lbs-ft	689 lbs-ft	689 lbs-ft	689 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft
e	0.10 ft	0.10 ft	0.09 ft	0.09 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.29 ft	1.82 ft	1.50 ft	1.29 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	269.1 psf	266.8 psf	264.8 psf	262.9 psf	240.0 psf	239.0 psf	238.1 psf	237.3 psf	269.1 psf	266.9 psf	264.8 psf	262.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	343.2 psf	337.7 psf	332.7 psf	328.1 psf	412.5 psf	404.0 psf	396.2 psf	389.1 psf	449.4 psf	439.3 psf	430.1 psf	421.6 psf	357.9 psf	133.9 psf	106.5 psf	98.1 psf

Maximum Bearing Pressure = 449 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

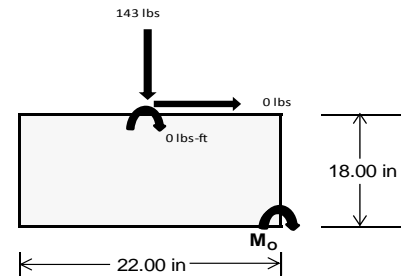
Overturning Check

$M_o = 0.0 \text{ ft-lbs}$
 Resisting Force Required = 0.00 lbs
 S.F. = 1.67
 Weight Required = 0.00 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	55 lbs	143 lbs	52 lbs	218 lbs	659 lbs	215 lbs	16 lbs	42 lbs	15 lbs
F_v	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2523 lbs	2611 lbs	2520 lbs	2567 lbs	3009 lbs	2564 lbs	738 lbs	764 lbs	737 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	275.2 sqft	284.8 sqft	274.9 sqft	279.8 sqft	328.0 sqft	279.6 sqft	80.5 sqft	83.3 sqft	80.4 sqft
f_{max}	275.3 psf	284.9 psf	274.9 psf	280.4 psf	328.4 psf	279.9 psf	80.5 psf	83.3 psf	80.4 psf



Maximum Bearing Pressure = 328 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

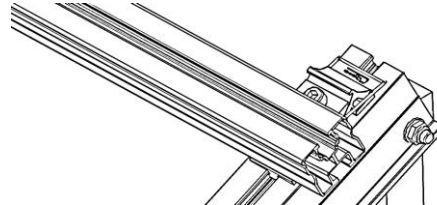
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.694 k
Allowable Uplift =	1.214 k
Utilization =	<u>57%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.110 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.139 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Diagonal Strut

Maximum Axial Load =	0.190 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.166 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.060 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.005 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	113.022	2	245.743	2	.016	9	0	9	0	1	0	1
2		min	-141.263	3	-364.629	3	-.178	3	0	3	0	1	0	1
3	N7	max	0	15	377.194	1	.003	10	0	10	0	1	0	1
4		min	-.113	2	-118.424	3	-.421	1	0	1	0	1	0	1
5	N15	max	0	15	1138.631	1	.134	9	0	1	0	1	0	1
6		min	-1.19	2	-405.097	3	-.419	3	0	3	0	1	0	1
7	N16	max	386.349	2	820.662	1	0	10	0	9	0	1	0	1
8		min	-431.219	3	-1178.553	3	-51.571	3	0	3	0	1	0	1
9	N23	max	0	15	377.323	1	.73	1	.001	1	0	1	0	1
10		min	-.113	2	-118.064	3	-.002	10	0	10	0	1	0	1
11	N24	max	113.023	2	248.248	2	52.032	3	0	1	0	1	0	1
12		min	-141.511	3	-363.331	3	-.003	10	0	3	0	1	0	1
13	Totals:	max	610.977	2	3203.51	1	0	3						
14		min	-714.372	3	-2548.099	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	271.359	1	.668	4	.199	1	0	10	0	3	0	1
2			min	-366.467	3	.158	15	-.124	3	0	1	0	2	0	1
3		2	max	271.456	1	.63	4	.199	1	0	10	0	1	0	15
4			min	-366.395	3	.149	15	-.124	3	0	1	0	10	0	4
5		3	max	271.552	1	.592	4	.199	1	0	10	0	1	0	15
6			min	-366.323	3	.14	15	-.124	3	0	1	0	3	0	4
7		4	max	271.648	1	.554	4	.199	1	0	10	0	1	0	15
8			min	-366.251	3	.131	15	-.124	3	0	1	0	3	0	4
9		5	max	271.745	1	.516	4	.199	1	0	10	0	1	0	15
10			min	-366.178	3	.122	15	-.124	3	0	1	0	3	0	4
11		6	max	271.841	1	.479	4	.199	1	0	10	0	1	0	15
12			min	-366.106	3	.113	15	-.124	3	0	1	0	3	0	4
13		7	max	271.938	1	.441	4	.199	1	0	10	0	1	0	15
14			min	-366.034	3	.105	15	-.124	3	0	1	0	3	0	4
15		8	max	272.034	1	.403	4	.199	1	0	10	0	1	0	15
16			min	-365.962	3	.096	15	-.124	3	0	1	0	3	0	4
17		9	max	272.13	1	.365	4	.199	1	0	10	0	1	0	15
18			min	-365.889	3	.087	15	-.124	3	0	1	0	3	0	4
19		10	max	272.227	1	.327	4	.199	1	0	10	0	1	0	15
20			min	-365.817	3	.078	15	-.124	3	0	1	0	3	0	4
21		11	max	272.323	1	.289	4	.199	1	0	10	0	1	0	15
22			min	-365.745	3	.069	15	-.124	3	0	1	0	3	0	4
23		12	max	272.419	1	.252	4	.199	1	0	10	0	1	0	15
24			min	-365.672	3	.06	15	-.124	3	0	1	0	3	0	4
25		13	max	272.516	1	.214	4	.199	1	0	10	0	1	0	15
26			min	-365.6	3	.051	15	-.124	3	0	1	0	3	0	4
27		14	max	272.612	1	.176	4	.199	1	0	10	0	1	0	15
28			min	-365.528	3	.042	15	-.124	3	0	1	0	3	0	4
29		15	max	272.708	1	.138	4	.199	1	0	10	0	1	0	15
30			min	-365.456	3	.033	15	-.124	3	0	1	0	3	0	4
31		16	max	272.805	1	.1	4	.199	1	0	10	0	1	0	15
32			min	-365.383	3	.024	15	-.124	3	0	1	0	3	0	4
33		17	max	272.901	1	.064	2	.199	1	0	10	0	1	0	15
34			min	-365.311	3	.016	15	-.124	3	0	1	0	3	0	4
35		18	max	272.998	1	.035	2	.199	1	0	10	0	1	0	15
36			min	-365.239	3	0	9	-.124	3	0	1	0	3	0	4
37		19	max	273.094	1	.008	10	.199	1	0	10	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-365.167	3	-.027	1	-.124	3	0	1	0	3	0	4
39	M3	1	max	50.732	2	1.816	4	-.002	10	0	10	0	1	4
40		min	-55.49	9	.428	15	-.186	1	0	1	0	10	0	15
41		2	max	50.664	2	1.638	4	-.002	10	0	10	0	1	4
42		min	-55.546	9	.386	15	-.186	1	0	1	0	10	0	15
43		3	max	50.597	2	1.46	4	-.002	10	0	10	0	1	2
44		min	-55.602	9	.344	15	-.186	1	0	1	0	10	0	15
45		4	max	50.53	2	1.282	4	-.002	10	0	10	0	1	15
46		min	-55.658	9	.302	15	-.186	1	0	1	0	10	0	4
47		5	max	50.463	2	1.104	4	-.002	10	0	10	0	1	15
48		min	-55.714	9	.26	15	-.186	1	0	1	0	10	0	4
49		6	max	50.396	2	.926	4	-.002	10	0	10	0	1	15
50		min	-55.77	9	.218	15	-.186	1	0	1	0	10	0	4
51		7	max	50.329	2	.748	4	-.002	10	0	10	0	1	15
52		min	-55.826	9	.176	15	-.186	1	0	1	0	10	0	4
53		8	max	50.262	2	.57	4	-.002	10	0	10	0	1	15
54		min	-55.882	9	.135	15	-.186	1	0	1	0	10	0	4
55		9	max	50.195	2	.392	4	-.002	10	0	10	0	1	15
56		min	-55.938	9	.093	15	-.186	1	0	1	0	10	-.001	4
57		10	max	50.128	2	.214	4	-.002	10	0	10	0	1	15
58		min	-55.994	9	.051	15	-.186	1	0	1	0	10	-.001	4
59		11	max	50.061	2	.038	2	-.002	10	0	10	0	1	15
60		min	-56.049	9	.009	15	-.186	1	0	1	0	10	-.001	4
61		12	max	49.994	2	-.033	15	-.002	10	0	10	0	1	15
62		min	-56.105	9	-.142	4	-.186	1	0	1	0	10	-.001	4
63		13	max	49.926	2	-.075	15	-.002	10	0	10	0	1	15
64		min	-56.161	9	-.32	4	-.186	1	0	1	0	10	-.001	4
65		14	max	49.859	2	-.116	15	-.002	10	0	10	0	9	15
66		min	-56.217	9	-.498	4	-.186	1	0	1	0	2	-.001	4
67		15	max	49.792	2	-.158	15	-.002	10	0	10	0	10	15
68		min	-56.273	9	-.676	4	-.186	1	0	1	0	1	0	4
69		16	max	49.725	2	-.2	15	-.002	10	0	10	0	10	15
70		min	-56.329	9	-.854	4	-.186	1	0	1	0	1	0	4
71		17	max	49.658	2	-.242	15	-.002	10	0	10	0	10	15
72		min	-56.385	9	-1.032	4	-.186	1	0	1	0	1	0	4
73		18	max	49.591	2	-.284	15	-.002	10	0	10	0	10	15
74		min	-56.441	9	-1.21	4	-.186	1	0	1	0	1	0	4
75		19	max	49.524	2	-.326	15	-.002	10	0	10	0	10	1
76		min	-56.497	9	-1.388	4	-.186	1	0	1	0	1	0	1
77	M4	1	max	376.03	1	0	1	.003	10	0	1	0	3	1
78		min	-119.298	3	0	1	-.45	1	0	1	0	2	0	1
79		2	max	376.094	1	0	1	.003	10	0	1	0	15	1
80		min	-119.249	3	0	1	-.45	1	0	1	0	1	0	1
81		3	max	376.159	1	0	1	.003	10	0	1	0	15	1
82		min	-119.201	3	0	1	-.45	1	0	1	0	1	0	1
83		4	max	376.224	1	0	1	.003	10	0	1	0	15	1
84		min	-119.152	3	0	1	-.45	1	0	1	0	1	0	1
85		5	max	376.288	1	0	1	.003	10	0	1	0	15	1
86		min	-119.104	3	0	1	-.45	1	0	1	0	1	0	1
87		6	max	376.353	1	0	1	.003	10	0	1	0	15	1
88		min	-119.055	3	0	1	-.45	1	0	1	0	1	0	1
89		7	max	376.418	1	0	1	.003	10	0	1	0	10	1
90		min	-119.007	3	0	1	-.45	1	0	1	0	1	0	1
91		8	max	376.482	1	0	1	.003	10	0	1	0	10	1
92		min	-118.958	3	0	1	-.45	1	0	1	0	1	0	1
93		9	max	376.547	1	0	1	.003	10	0	1	0	10	1
94		min	-118.91	3	0	1	-.45	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	376.612	1	0	1	.003	10	0	1	0	10	0	1
96		min	-118.861	3	0	1	-.45	1	0	1	0	1	0	1
97	11	max	376.677	1	0	1	.003	10	0	1	0	10	0	1
98		min	-118.813	3	0	1	-.45	1	0	1	0	1	0	1
99	12	max	376.741	1	0	1	.003	10	0	1	0	10	0	1
100		min	-118.764	3	0	1	-.45	1	0	1	0	1	0	1
101	13	max	376.806	1	0	1	.003	10	0	1	0	10	0	1
102		min	-118.715	3	0	1	-.45	1	0	1	0	1	0	1
103	14	max	376.871	1	0	1	.003	10	0	1	0	10	0	1
104		min	-118.667	3	0	1	-.45	1	0	1	0	1	0	1
105	15	max	376.935	1	0	1	.003	10	0	1	0	10	0	1
106		min	-118.618	3	0	1	-.45	1	0	1	0	1	0	1
107	16	max	377	1	0	1	.003	10	0	1	0	10	0	1
108		min	-118.57	3	0	1	-.45	1	0	1	0	1	0	1
109	17	max	377.065	1	0	1	.003	10	0	1	0	10	0	1
110		min	-118.521	3	0	1	-.45	1	0	1	0	1	0	1
111	18	max	377.13	1	0	1	.003	10	0	1	0	10	0	1
112		min	-118.473	3	0	1	-.45	1	0	1	0	1	0	1
113	19	max	377.194	1	0	1	.003	10	0	1	0	10	0	1
114		min	-118.424	3	0	1	-.45	1	0	1	0	1	0	1
115	M6	1	max 867.291	1	.656	4	.055	9	0	3	0	3	0	1
116		min	-1166.497	3	.156	15	-.256	3	0	2	0	1	0	1
117	2	max	867.387	1	.618	4	.055	9	0	3	0	3	0	15
118		min	-1166.425	3	.147	15	-.256	3	0	2	0	2	0	4
119	3	max	867.483	1	.581	4	.055	9	0	3	0	3	0	15
120		min	-1166.353	3	.138	15	-.256	3	0	2	0	2	0	4
121	4	max	867.58	1	.543	4	.055	9	0	3	0	9	0	15
122		min	-1166.28	3	.13	15	-.256	3	0	2	0	2	0	4
123	5	max	867.676	1	.505	4	.055	9	0	3	0	9	0	15
124		min	-1166.208	3	.121	15	-.256	3	0	2	0	3	0	4
125	6	max	867.773	1	.467	4	.055	9	0	3	0	9	0	15
126		min	-1166.136	3	.112	15	-.256	3	0	2	0	3	0	4
127	7	max	867.869	1	.429	4	.055	9	0	3	0	9	0	15
128		min	-1166.064	3	.103	15	-.256	3	0	2	0	3	0	4
129	8	max	867.965	1	.391	4	.055	9	0	3	0	9	0	15
130		min	-1165.991	3	.094	15	-.256	3	0	2	0	3	0	4
131	9	max	868.062	1	.354	4	.055	9	0	3	0	9	0	15
132		min	-1165.919	3	.085	15	-.256	3	0	2	0	3	0	4
133	10	max	868.158	1	.316	4	.055	9	0	3	0	9	0	15
134		min	-1165.847	3	.076	15	-.256	3	0	2	0	3	0	4
135	11	max	868.254	1	.278	4	.055	9	0	3	0	9	0	15
136		min	-1165.774	3	.067	15	-.256	3	0	2	0	3	0	4
137	12	max	868.351	1	.241	2	.055	9	0	3	0	9	0	15
138		min	-1165.702	3	.058	15	-.256	3	0	2	0	3	0	4
139	13	max	868.447	1	.211	2	.055	9	0	3	0	9	0	15
140		min	-1165.63	3	.05	15	-.256	3	0	2	0	3	0	4
141	14	max	868.544	1	.182	2	.055	9	0	3	0	9	0	15
142		min	-1165.558	3	.041	15	-.256	3	0	2	0	3	0	4
143	15	max	868.64	1	.152	2	.055	9	0	3	0	9	0	15
144		min	-1165.485	3	.032	15	-.256	3	0	2	0	3	0	4
145	16	max	868.736	1	.123	2	.055	9	0	3	0	9	0	15
146		min	-1165.413	3	.021	9	-.256	3	0	2	0	3	0	4
147	17	max	868.833	1	.093	2	.055	9	0	3	0	9	0	15
148		min	-1165.341	3	-.003	9	-.256	3	0	2	0	3	0	4
149	18	max	868.929	1	.064	2	.055	9	0	3	0	9	0	15
150		min	-1165.269	3	-.028	9	-.256	3	0	2	0	3	0	4
151	19	max	869.025	1	.034	2	.055	9	0	3	0	9	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1165.196	3	-.052	9	-.256	3	0	2	0	3	0	4
153	M7	1	max	189.885	2	1.812	4	0	13	0	1	0	1	4
154		min	-100.03	9	.427	15	-.012	1	0	3	0	3	0	15
155		2	max	189.818	2	1.634	4	0	13	0	1	0	1	2
156		min	-100.085	9	.385	15	-.012	1	0	3	0	3	0	15
157		3	max	189.751	2	1.456	4	0	13	0	1	0	1	2
158		min	-100.141	9	.343	15	-.012	1	0	3	0	3	0	9
159		4	max	189.684	2	1.278	4	0	13	0	1	0	1	10
160		min	-100.197	9	.301	15	-.012	1	0	3	0	3	0	9
161		5	max	189.616	2	1.1	4	0	13	0	1	0	1	15
162		min	-100.253	9	.26	15	-.012	1	0	3	0	3	0	4
163		6	max	189.549	2	.922	4	0	13	0	1	0	1	15
164		min	-100.309	9	.218	15	-.012	1	0	3	0	3	0	4
165		7	max	189.482	2	.743	4	0	13	0	1	0	1	15
166		min	-100.365	9	.176	15	-.012	1	0	3	0	3	0	4
167		8	max	189.415	2	.565	4	0	13	0	1	0	1	15
168		min	-100.421	9	.134	15	-.012	1	0	3	0	3	0	4
169		9	max	189.348	2	.387	4	0	13	0	1	0	1	15
170		min	-100.477	9	.092	15	-.012	1	0	3	0	3	-.001	4
171		10	max	189.281	2	.209	4	0	13	0	1	0	1	15
172		min	-100.533	9	.05	15	-.012	1	0	3	0	3	-.001	4
173		11	max	189.214	2	.057	2	0	13	0	1	0	1	15
174		min	-100.589	9	-.001	9	-.012	1	0	3	0	3	-.001	4
175		12	max	189.147	2	-.033	15	0	13	0	1	0	1	15
176		min	-100.645	9	-.147	4	-.012	1	0	3	0	3	-.001	4
177		13	max	189.08	2	-.075	15	0	13	0	1	0	1	15
178		min	-100.7	9	-.325	4	-.012	1	0	3	0	3	-.001	4
179		14	max	189.013	2	-.117	15	0	13	0	1	0	1	15
180		min	-100.756	9	-.503	4	-.012	1	0	3	0	3	-.001	4
181		15	max	188.945	2	-.159	15	0	13	0	1	0	1	15
182		min	-100.812	9	-.681	4	-.012	1	0	3	0	3	0	4
183		16	max	188.878	2	-.201	15	0	13	0	1	0	1	15
184		min	-100.868	9	-.859	4	-.012	1	0	3	0	3	0	4
185		17	max	188.811	2	-.243	15	0	13	0	1	0	1	15
186		min	-100.924	9	-1.037	4	-.012	1	0	3	0	3	0	4
187		18	max	188.744	2	-.284	15	0	13	0	1	0	1	15
188		min	-100.98	9	-1.215	4	-.012	1	0	3	0	3	0	4
189		19	max	188.677	2	-.326	15	0	13	0	1	0	1	1
190		min	-101.036	9	-1.393	4	-.012	1	0	3	0	3	0	1
191	M8	1	max	1137.466	1	0	1	.161	1	0	1	0	2	1
192		min	-405.971	3	0	1	-.393	3	0	1	0	1	0	1
193		2	max	1137.531	1	0	1	.161	1	0	1	0	1	1
194		min	-405.922	3	0	1	-.393	3	0	1	0	3	0	1
195		3	max	1137.596	1	0	1	.161	1	0	1	0	1	1
196		min	-405.874	3	0	1	-.393	3	0	1	0	3	0	1
197		4	max	1137.661	1	0	1	.161	1	0	1	0	1	1
198		min	-405.825	3	0	1	-.393	3	0	1	0	3	0	1
199		5	max	1137.725	1	0	1	.161	1	0	1	0	1	1
200		min	-405.777	3	0	1	-.393	3	0	1	0	3	0	1
201		6	max	1137.79	1	0	1	.161	1	0	1	0	1	1
202		min	-405.728	3	0	1	-.393	3	0	1	0	3	0	1
203		7	max	1137.855	1	0	1	.161	1	0	1	0	1	1
204		min	-405.679	3	0	1	-.393	3	0	1	0	3	0	1
205		8	max	1137.919	1	0	1	.161	1	0	1	0	1	1
206		min	-405.631	3	0	1	-.393	3	0	1	0	3	0	1
207		9	max	1137.984	1	0	1	.161	1	0	1	0	1	1
208		min	-405.582	3	0	1	-.393	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1138.049	1	0	1	.161	1	0	1	0	1	0	1
210			min	-405.534	3	0	1	-.393	3	0	1	0	3	0	1
211		11	max	1138.114	1	0	1	.161	1	0	1	0	1	0	1
212			min	-405.485	3	0	1	-.393	3	0	1	0	3	0	1
213		12	max	1138.178	1	0	1	.161	1	0	1	0	1	0	1
214			min	-405.437	3	0	1	-.393	3	0	1	0	3	0	1
215		13	max	1138.243	1	0	1	.161	1	0	1	0	1	0	1
216			min	-405.388	3	0	1	-.393	3	0	1	0	3	0	1
217		14	max	1138.308	1	0	1	.161	1	0	1	0	1	0	1
218			min	-405.34	3	0	1	-.393	3	0	1	0	3	0	1
219		15	max	1138.372	1	0	1	.161	1	0	1	0	1	0	1
220			min	-405.291	3	0	1	-.393	3	0	1	0	3	0	1
221		16	max	1138.437	1	0	1	.161	1	0	1	0	1	0	1
222			min	-405.243	3	0	1	-.393	3	0	1	0	3	0	1
223		17	max	1138.502	1	0	1	.161	1	0	1	0	1	0	1
224			min	-405.194	3	0	1	-.393	3	0	1	0	3	0	1
225		18	max	1138.567	1	0	1	.161	1	0	1	0	1	0	1
226			min	-405.146	3	0	1	-.393	3	0	1	0	3	0	1
227		19	max	1138.631	1	0	1	.161	1	0	1	0	1	0	1
228			min	-405.097	3	0	1	-.393	3	0	1	0	3	0	1
229	M10	1	max	272.991	1	.668	4	-.002	15	0	1	0	1	0	1
230			min	-336.832	3	.158	15	-.098	1	0	3	0	3	0	1
231		2	max	273.087	1	.63	4	-.002	15	0	1	0	1	0	15
232			min	-336.759	3	.149	15	-.098	1	0	3	0	3	0	4
233		3	max	273.184	1	.592	4	-.002	15	0	1	0	1	0	15
234			min	-336.687	3	.14	15	-.098	1	0	3	0	3	0	4
235		4	max	273.28	1	.554	4	-.002	15	0	1	0	10	0	15
236			min	-336.615	3	.131	15	-.098	1	0	3	0	3	0	4
237		5	max	273.376	1	.516	4	-.002	15	0	1	0	10	0	15
238			min	-336.542	3	.122	15	-.098	1	0	3	0	3	0	4
239		6	max	273.473	1	.478	4	-.002	15	0	1	0	10	0	15
240			min	-336.47	3	.113	15	-.098	1	0	3	0	3	0	4
241		7	max	273.569	1	.441	4	-.002	15	0	1	0	10	0	15
242			min	-336.398	3	.104	15	-.098	1	0	3	0	3	0	4
243		8	max	273.665	1	.403	4	-.002	15	0	1	0	10	0	15
244			min	-336.326	3	.096	15	-.098	1	0	3	0	3	0	4
245		9	max	273.762	1	.365	4	-.002	15	0	1	0	10	0	15
246			min	-336.253	3	.087	15	-.098	1	0	3	0	3	0	4
247		10	max	273.858	1	.327	4	-.002	15	0	1	0	10	0	15
248			min	-336.181	3	.078	15	-.098	1	0	3	0	3	0	4
249		11	max	273.955	1	.289	4	-.002	15	0	1	0	10	0	15
250			min	-336.109	3	.069	15	-.098	1	0	3	0	3	0	4
251		12	max	274.051	1	.251	4	-.002	15	0	1	0	10	0	15
252			min	-336.037	3	.06	15	-.098	1	0	3	0	3	0	4
253		13	max	274.147	1	.214	4	-.002	15	0	1	0	10	0	15
254			min	-335.964	3	.051	15	-.098	1	0	3	0	3	0	4
255		14	max	274.244	1	.176	4	-.002	15	0	1	0	10	0	15
256			min	-335.892	3	.042	15	-.098	1	0	3	0	3	0	4
257		15	max	274.34	1	.138	4	-.002	15	0	1	0	15	0	15
258			min	-335.82	3	.033	15	-.098	1	0	3	0	3	0	4
259		16	max	274.436	1	.1	4	-.002	15	0	1	0	15	0	15
260			min	-335.747	3	.024	15	-.098	1	0	3	0	3	0	4
261		17	max	274.533	1	.078	3	-.002	15	0	1	0	15	0	15
262			min	-335.675	3	.016	15	-.098	1	0	3	0	3	0	4
263		18	max	274.629	1	.056	3	-.002	15	0	1	0	15	0	15
264			min	-335.603	3	-.001	9	-.098	1	0	3	0	3	0	4
265		19	max	274.725	1	.034	3	-.002	15	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-335.531	3	-.027	1	-.098	1	0	3	0	3	0	4
267		1	max	50.288	2	1.816	4	.203	1	0	3	0	3	0	4
268			min	-55.573	9	.427	15	-.017	3	0	10	0	1	0	15
269		2	max	50.221	2	1.638	4	.203	1	0	3	0	3	0	4
270			min	-55.629	9	.386	15	-.017	3	0	10	0	1	0	15
271		3	max	50.154	2	1.46	4	.203	1	0	3	0	3	0	2
272			min	-55.685	9	.344	15	-.017	3	0	10	0	1	0	3
273		4	max	50.087	2	1.282	4	.203	1	0	3	0	3	0	15
274			min	-55.741	9	.302	15	-.017	3	0	10	0	1	0	4
275		5	max	50.02	2	1.104	4	.203	1	0	3	0	3	0	15
276			min	-55.797	9	.26	15	-.017	3	0	10	0	1	0	4
277		6	max	49.953	2	.926	4	.203	1	0	3	0	3	0	15
278			min	-55.853	9	.218	15	-.017	3	0	10	0	1	0	4
279		7	max	49.886	2	.748	4	.203	1	0	3	0	3	0	15
280			min	-55.909	9	.176	15	-.017	3	0	10	0	1	0	4
281	8	max	49.819	2	.57	4	.203	1	0	3	0	3	0	15	
282		min	-55.965	9	.135	15	-.017	3	0	10	0	1	0	4	
283	9	max	49.752	2	.392	4	.203	1	0	3	0	3	0	15	
284		min	-56.021	9	.093	15	-.017	3	0	10	0	1	-.001	4	
285	10	max	49.684	2	.214	4	.203	1	0	3	0	3	0	15	
286		min	-56.077	9	.051	15	-.017	3	0	10	0	1	-.001	4	
287	11	max	49.617	2	.038	2	.203	1	0	3	0	3	0	15	
288		min	-56.133	9	0	3	-.017	3	0	10	0	1	-.001	4	
289	12	max	49.55	2	-.033	15	.203	1	0	3	0	3	0	15	
290		min	-56.188	9	-.142	4	-.017	3	0	10	0	1	-.001	4	
291	13	max	49.483	2	-.075	15	.203	1	0	3	0	3	0	15	
292		min	-56.244	9	-.32	4	-.017	3	0	10	0	2	-.001	4	
293	14	max	49.416	2	-.117	15	.203	1	0	3	0	3	0	15	
294		min	-56.3	9	-.498	4	-.017	3	0	10	0	10	-.001	4	
295	15	max	49.349	2	-.158	15	.203	1	0	3	0	3	0	15	
296		min	-56.356	9	-.676	4	-.017	3	0	10	0	10	0	4	
297	16	max	49.282	2	-.2	15	.203	1	0	3	0	3	0	15	
298		min	-56.412	9	-.854	4	-.017	3	0	10	0	10	0	4	
299	17	max	49.215	2	-.242	15	.203	1	0	3	0	3	0	15	
300		min	-56.468	9	-1.032	4	-.017	3	0	10	0	10	0	4	
301	18	max	49.148	2	-.284	15	.203	1	0	3	0	3	0	15	
302		min	-56.524	9	-1.21	4	-.017	3	0	10	0	10	0	4	
303	19	max	49.081	2	-.326	15	.203	1	0	3	0	3	0	1	
304		min	-56.58	9	-1.388	4	-.017	3	0	10	0	10	0	1	
305	M12	1	max	376.158	1	0	1	.778	1	0	1	0	2	0	1
306			min	-118.938	3	0	1	-.002	10	0	1	0	3	0	1
307		2	max	376.223	1	0	1	.778	1	0	1	0	1	0	1
308			min	-118.889	3	0	1	-.002	10	0	1	0	15	0	1
309		3	max	376.288	1	0	1	.778	1	0	1	0	1	0	1
310			min	-118.841	3	0	1	-.002	10	0	1	0	15	0	1
311		4	max	376.353	1	0	1	.778	1	0	1	0	1	0	1
312			min	-118.792	3	0	1	-.002	10	0	1	0	15	0	1
313		5	max	376.417	1	0	1	.778	1	0	1	0	1	0	1
314			min	-118.744	3	0	1	-.002	10	0	1	0	10	0	1
315		6	max	376.482	1	0	1	.778	1	0	1	0	1	0	1
316			min	-118.695	3	0	1	-.002	10	0	1	0	10	0	1
317		7	max	376.547	1	0	1	.778	1	0	1	0	1	0	1
318			min	-118.647	3	0	1	-.002	10	0	1	0	10	0	1
319		8	max	376.611	1	0	1	.778	1	0	1	0	1	0	1
320			min	-118.598	3	0	1	-.002	10	0	1	0	10	0	1
321	9	max	376.676	1	0	1	.778	1	0	1	0	1	0	1	
322		min	-118.55	3	0	1	-.002	10	0	1	0	10	0	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	376.741	1	0	1	.778	1	0	1	0	1	0	1
324		min	-118.501	3	0	1	-.002	10	0	1	0	10	0	1
325	11	max	376.806	1	0	1	.778	1	0	1	0	1	0	1
326		min	-118.452	3	0	1	-.002	10	0	1	0	10	0	1
327	12	max	376.87	1	0	1	.778	1	0	1	0	1	0	1
328		min	-118.404	3	0	1	-.002	10	0	1	0	10	0	1
329	13	max	376.935	1	0	1	.778	1	0	1	0	1	0	1
330		min	-118.355	3	0	1	-.002	10	0	1	0	10	0	1
331	14	max	377	1	0	1	.778	1	0	1	0	1	0	1
332		min	-118.307	3	0	1	-.002	10	0	1	0	10	0	1
333	15	max	377.064	1	0	1	.778	1	0	1	0	1	0	1
334		min	-118.258	3	0	1	-.002	10	0	1	0	10	0	1
335	16	max	377.129	1	0	1	.778	1	0	1	.001	1	0	1
336		min	-118.21	3	0	1	-.002	10	0	1	0	10	0	1
337	17	max	377.194	1	0	1	.778	1	0	1	.001	1	0	1
338		min	-118.161	3	0	1	-.002	10	0	1	0	10	0	1
339	18	max	377.259	1	0	1	.778	1	0	1	.001	1	0	1
340		min	-118.113	3	0	1	-.002	10	0	1	0	10	0	1
341	19	max	377.323	1	0	1	.778	1	0	1	.001	1	0	1
342		min	-118.064	3	0	1	-.002	10	0	1	0	10	0	1
343	M1	1	max	59.716	1	344.8	3	-.178	10	0	.033	1	0	1
344		min	1.846	15	-273.939	1	-16.993	1	0	3	0	10	0	3
345	2	max	59.788	1	344.598	3	-.178	10	0	1	.03	1	.06	1
346		min	1.868	15	-274.209	1	-16.993	1	0	3	0	10	-.075	3
347	3	max	69.838	1	4.418	9	-.174	10	0	3	.026	1	.118	1
348		min	-6.663	3	-21.881	3	-16.854	1	0	1	0	10	-.148	3
349	4	max	69.91	1	4.193	9	-.174	10	0	3	.022	1	.119	1
350		min	-6.609	3	-22.083	3	-16.854	1	0	1	0	10	-.144	3
351	5	max	69.983	1	3.968	9	-.174	10	0	3	.018	1	.12	1
352		min	-6.555	3	-22.285	3	-16.854	1	0	1	0	10	-.139	3
353	6	max	70.055	1	3.744	9	-.174	10	0	3	.015	1	.12	1
354		min	-6.5	3	-22.487	3	-16.854	1	0	1	0	10	-.134	3
355	7	max	70.127	1	3.519	9	-.174	10	0	3	.011	1	.121	1
356		min	-6.446	3	-22.69	3	-16.854	1	0	1	0	10	-.129	3
357	8	max	70.199	1	3.294	9	-.174	10	0	3	.007	1	.123	2
358		min	-6.392	3	-22.892	3	-16.854	1	0	1	0	10	-.124	3
359	9	max	70.272	1	3.069	9	-.174	10	0	3	.004	1	.127	2
360		min	-6.338	3	-23.094	3	-16.854	1	0	1	0	10	-.119	3
361	10	max	70.344	1	2.844	9	-.174	10	0	3	.001	3	.13	2
362		min	-6.284	3	-23.297	3	-16.854	1	0	1	0	15	-.114	3
363	11	max	70.416	1	2.62	9	-.174	10	0	3	0	3	.134	2
364		min	-6.229	3	-23.499	3	-16.854	1	0	1	-.004	1	-.109	3
365	12	max	70.489	1	2.395	9	-.174	10	0	3	0	12	.138	2
366		min	-6.175	3	-23.701	3	-16.854	1	0	1	-.007	1	-.104	3
367	13	max	70.561	1	2.17	9	-.174	10	0	3	0	10	.142	2
368		min	-6.121	3	-23.903	3	-16.854	1	0	1	-.011	1	-.099	3
369	14	max	70.633	1	1.945	9	-.174	10	0	3	0	10	.145	2
370		min	-6.067	3	-24.106	3	-16.854	1	0	1	-.014	1	-.093	3
371	15	max	70.705	1	1.721	9	-.174	10	0	3	0	10	.149	2
372		min	-6.013	3	-24.308	3	-16.854	1	0	1	-.018	1	-.088	3
373	16	max	68.986	2	15.303	10	-.176	10	0	1	0	10	.153	2
374		min	-34.641	3	-50.262	3	-17.027	1	0	10	-.022	1	-.083	3
375	17	max	69.059	2	15.079	10	-.176	10	0	1	0	10	.15	2
376		min	-34.587	3	-50.465	3	-17.027	1	0	10	-.026	1	-.072	3
377	18	max	-1.867	15	348.965	2	-.173	10	0	3	0	10	.076	2
378		min	-59.746	1	-165.979	3	-17.478	1	0	2	-.03	1	-.036	3
379	19	max	-1.845	15	348.696	2	-.173	10	0	3	0	10	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-59.674	1	-166.182	3	-17.478	1	0	2	-.033	1	0	3
381	M5	1	max	142.8	1	1119.426	3	0	10	0	.006	3	0	3
382		min	.632	3	-886.309	1	-46.617	3	0	3	0	10	0	1
383		2	max	142.872	1	1119.224	3	0	10	0	0	9	.192	1
384		min	.686	3	-886.578	1	-46.617	3	0	3	-.004	3	-.242	3
385		3	max	173.772	1	6.727	9	5.109	3	0	0	1	.381	1
386		min	-40.919	3	-76.809	3	-.164	9	0	1	-.013	3	-.48	3
387		4	max	173.844	1	6.502	9	5.109	3	0	0	1	.385	1
388		min	-40.865	3	-77.011	3	-.164	9	0	1	-.012	3	-.463	3
389		5	max	173.916	1	6.278	9	5.109	3	0	0	1	.389	1
390		min	-40.811	3	-77.213	3	-.164	9	0	1	-.011	3	-.446	3
391		6	max	173.988	1	6.053	9	5.109	3	0	0	1	.394	1
392		min	-40.757	3	-77.416	3	-.164	9	0	1	-.01	3	-.43	3
393		7	max	174.061	1	5.828	9	5.109	3	0	0	1	.398	1
394		min	-40.702	3	-77.618	3	-.164	9	0	1	-.009	3	-.413	3
395		8	max	174.133	1	5.603	9	5.109	3	0	0	1	.403	1
396		min	-40.648	3	-77.82	3	-.164	9	0	1	-.008	3	-.396	3
397		9	max	174.205	1	5.378	9	5.109	3	0	0	1	.413	2
398		min	-40.594	3	-78.023	3	-.164	9	0	1	-.007	3	-.379	3
399		10	max	174.277	1	5.154	9	5.109	3	0	0	2	.425	2
400		min	-40.54	3	-78.225	3	-.164	9	0	1	-.006	3	-.362	3
401		11	max	174.35	1	4.929	9	5.109	3	0	0	2	.437	2
402		min	-40.486	3	-78.427	3	-.164	9	0	1	-.004	3	-.345	3
403		12	max	174.422	1	4.704	9	5.109	3	0	0	2	.45	2
404		min	-40.431	3	-78.629	3	-.164	9	0	1	-.003	3	-.328	3
405		13	max	174.494	1	4.479	9	5.109	3	0	0	2	.462	2
406		min	-40.377	3	-78.832	3	-.164	9	0	1	-.002	3	-.311	3
407		14	max	174.567	1	4.255	9	5.109	3	0	0	2	.475	2
408		min	-40.323	3	-79.034	3	-.164	9	0	1	-.001	3	-.294	3
409		15	max	174.639	1	4.03	9	5.109	3	0	0	3	.487	2
410		min	-40.269	3	-79.236	3	-.164	9	0	1	0	9	-.277	3
411		16	max	228.615	2	66.559	2	5.083	3	0	0	3	.499	2
412		min	-108.584	3	-138.67	3	-.174	1	0	2	0	9	-.259	3
413		17	max	228.687	2	66.289	2	5.083	3	0	.002	3	.484	2
414		min	-108.53	3	-138.872	3	-.174	1	0	2	0	9	-.229	3
415		18	max	-2.579	12	1128.101	2	4.685	3	0	.003	3	.244	2
416		min	-142.964	1	-532.072	3	-.041	1	0	1	0	9	-.115	3
417		19	max	-2.542	12	1127.832	2	4.685	3	0	.004	3	0	3
418		min	-142.891	1	-532.274	3	-.041	1	0	1	0	9	0	2
419	M9	1	max	59.604	1	344.758	3	49.099	3	0	0	10	0	1
420		min	1.841	15	-273.938	1	.178	10	0	1	-.033	1	0	3
421		2	max	59.676	1	344.556	3	49.099	3	0	0	3	.06	1
422		min	1.862	15	-274.208	1	.178	10	0	1	-.029	1	-.075	3
423		3	max	70.094	1	4.404	9	16.569	1	0	.01	3	.118	1
424		min	-6.779	3	-21.802	3	-2.147	3	0	10	-.025	1	-.148	3
425		4	max	70.167	1	4.179	9	16.569	1	0	.01	3	.119	1
426		min	-6.725	3	-22.004	3	-2.147	3	0	10	-.022	1	-.143	3
427		5	max	70.239	1	3.954	9	16.569	1	0	.009	3	.119	1
428		min	-6.67	3	-22.206	3	-2.147	3	0	10	-.018	1	-.139	3
429		6	max	70.311	1	3.729	9	16.569	1	0	.009	3	.12	1
430		min	-6.616	3	-22.409	3	-2.147	3	0	10	-.014	1	-.134	3
431		7	max	70.383	1	3.505	9	16.569	1	0	.008	3	.121	1
432		min	-6.562	3	-22.611	3	-2.147	3	0	10	-.011	1	-.129	3
433		8	max	70.456	1	3.28	9	16.569	1	0	.008	3	.123	2
434		min	-6.508	3	-22.813	3	-2.147	3	0	10	-.007	1	-.124	3
435		9	max	70.528	1	3.055	9	16.569	1	0	.007	3	.127	2
436		min	-6.454	3	-23.016	3	-2.147	3	0	10	-.004	1	-.119	3



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	70.6	1	2.83	9	16.569	1	0	1	.007	3	.13	2
438		min	-6.399	3	-23.218	3	-2.147	3	0	10	0	1	-.114	3
439	11	max	70.672	1	2.606	9	16.569	1	0	1	.007	3	.134	2
440		min	-6.345	3	-23.42	3	-2.147	3	0	10	0	10	-.109	3
441	12	max	70.745	1	2.381	9	16.569	1	0	1	.007	1	.138	2
442		min	-6.291	3	-23.622	3	-2.147	3	0	10	0	10	-.104	3
443	13	max	70.817	1	2.156	9	16.569	1	0	1	.011	1	.142	2
444		min	-6.237	3	-23.825	3	-2.147	3	0	10	0	10	-.099	3
445	14	max	70.889	1	1.931	9	16.569	1	0	1	.014	1	.145	2
446		min	-6.183	3	-24.027	3	-2.147	3	0	10	0	10	-.093	3
447	15	max	70.961	1	1.706	9	16.569	1	0	1	.018	1	.149	2
448		min	-6.128	3	-24.229	3	-2.147	3	0	10	0	10	-.088	3
449	16	max	69.058	2	15.08	10	16.76	1	0	10	.022	1	.153	2
450		min	-35.218	3	-50.606	3	-2.155	3	0	1	0	10	-.083	3
451	17	max	69.131	2	14.855	10	16.76	1	0	10	.025	1	.15	2
452		min	-35.164	3	-50.808	3	-2.155	3	0	1	0	10	-.072	3
453	18	max	-1.861	15	348.966	2	17.521	1	0	2	.029	1	.076	2
454		min	-59.626	1	-165.974	3	-1.835	3	0	3	0	10	-.036	3
455	19	max	-1.84	15	348.696	2	17.521	1	0	2	.033	1	0	2
456		min	-59.554	1	-166.177	3	-1.835	3	0	3	0	10	0	3
457	M13	1	max	49.097	3	273.752	1	-1.841	15	0	.033	1	0	1
458		min	.178	10	-344.782	3	-59.601	1	0	3	0	10	0	3
459	2	max	49.097	3	194.185	1	-1.398	15	0	1	.008	3	.147	3
460		min	.178	10	-244.285	3	-45.039	1	0	3	-.001	10	-.117	1
461	3	max	49.097	3	114.617	1	-.956	15	0	1	.006	3	.244	3
462		min	.178	10	-143.789	3	-30.476	1	0	3	-.012	1	-.194	1
463	4	max	49.097	3	35.05	1	-.172	10	0	1	.004	3	.291	3
464		min	.178	10	-43.292	3	-15.914	1	0	3	-.024	1	-.232	1
465	5	max	49.097	3	57.204	3	1.09	2	0	1	.003	3	.288	3
466		min	.178	10	-44.518	1	-2.658	3	0	3	-.028	1	-.229	1
467	6	max	49.097	3	157.701	3	13.21	1	0	1	.001	3	.234	3
468		min	.178	10	-124.085	1	-2.014	3	0	3	-.025	1	-.187	1
469	7	max	49.097	3	258.197	3	27.773	1	0	1	0	3	.13	3
470		min	.178	10	-203.653	1	-1.371	3	0	3	-.015	1	-.105	1
471	8	max	49.097	3	358.694	3	42.335	1	0	1	.003	2	.017	1
472		min	.178	10	-283.22	1	-.727	3	0	3	0	12	-.024	3
473	9	max	49.097	3	459.19	3	56.897	1	0	1	.028	1	.178	1
474		min	.178	10	-362.787	1	-.083	3	0	3	0	3	-.229	3
475	10	max	49.097	3	559.687	3	71.459	1	0	1	.06	1	.379	1
476		min	.178	10	-442.355	1	.48	12	0	3	-.006	3	-.484	3
477	11	max	17.018	1	362.787	1	.582	3	0	3	.028	1	.178	1
478		min	.178	10	-459.19	3	-56.785	1	0	1	-.006	3	-.229	3
479	12	max	17.018	1	283.22	1	1.226	3	0	3	.003	2	.017	1
480		min	.178	10	-358.694	3	-42.223	1	0	1	-.006	3	-.024	3
481	13	max	17.018	1	203.653	1	1.87	3	0	3	0	10	.13	3
482		min	.178	10	-258.197	3	-27.66	1	0	1	-.015	1	-.105	1
483	14	max	17.018	1	124.085	1	2.514	3	0	3	0	15	.234	3
484		min	.178	10	-157.701	3	-13.098	1	0	1	-.025	1	-.187	1
485	15	max	17.018	1	44.518	1	3.157	3	0	3	0	15	.288	3
486		min	.178	10	-57.204	3	-1.09	2	0	1	-.028	1	-.229	1
487	16	max	17.018	1	43.292	3	16.026	1	0	3	0	12	.291	3
488		min	.178	10	-35.05	1	.173	10	0	1	-.023	1	-.232	1
489	17	max	17.018	1	143.789	3	30.588	1	0	3	.001	3	.244	3
490		min	.178	10	-114.617	1	.961	15	0	1	-.012	1	-.194	1
491	18	max	17.018	1	244.285	3	45.151	1	0	3	.007	1	.147	3
492		min	.178	10	-194.185	1	1.404	15	0	1	-.001	10	-.117	1
493	19	max	17.018	1	344.782	3	59.713	1	0	3	.033	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	.178	10	-273.752	1	1.846	15	0	1	0	10	0	3
495	M16	1	max	1.837	3	348.775	2	-1.84	15	0	3	.033	1	0	2
496			min	-17.496	1	-166.19	3	-59.558	1	0	2	0	10	0	3
497		2	max	1.837	3	247.323	2	-1.397	15	0	3	.007	1	.071	3
498			min	-17.496	1	-118.152	3	-44.995	1	0	2	-.001	10	-.149	2
499		3	max	1.837	3	145.871	2	-.955	15	0	3	0	12	.118	3
500			min	-17.496	1	-70.114	3	-30.433	1	0	2	-.012	1	-.247	2
501		4	max	1.837	3	44.419	2	-.158	10	0	3	0	15	.141	3
502			min	-17.496	1	-22.075	3	-15.871	1	0	2	-.024	1	-.295	2
503		5	max	1.837	3	25.963	3	1.115	2	0	3	0	15	.14	3
504			min	-17.496	1	-57.033	2	-1.727	3	0	2	-.028	1	-.292	2
505		6	max	1.837	3	74.001	3	13.254	1	0	3	0	15	.115	3
506			min	-17.496	1	-158.485	2	-1.083	3	0	2	-.025	1	-.238	2
507		7	max	1.837	3	122.039	3	27.816	1	0	3	0	10	.066	3
508			min	-17.496	1	-259.936	2	-.439	3	0	2	-.015	1	-.133	2
509		8	max	1.837	3	170.077	3	42.378	1	0	3	.003	2	.022	2
510			min	-17.496	1	-361.388	2	.204	12	0	2	-.004	3	-.007	3
511		9	max	1.837	3	218.115	3	56.94	1	0	3	.028	1	.228	2
512			min	-17.496	1	-462.84	2	.633	12	0	2	-.004	3	-.104	3
513		10	max	-.173	10	-9.183	15	71.503	1	0	15	.06	1	.485	2
514			min	-17.496	1	-564.292	2	-2.164	3	0	2	-.003	3	-.225	3
515		11	max	-.173	10	462.84	2	-1.051	12	0	2	.028	1	.228	2
516			min	-17.454	1	-218.115	3	-56.82	1	0	3	0	3	-.104	3
517		12	max	-.173	10	361.388	2	-.622	12	0	2	.003	2	.022	2
518			min	-17.454	1	-170.077	3	-42.258	1	0	3	0	3	-.007	3
519		13	max	-.173	10	259.936	2	-.193	12	0	2	0	10	.066	3
520			min	-17.454	1	-122.039	3	-27.696	1	0	3	-.015	1	-.133	2
521		14	max	-.173	10	158.485	2	.412	3	0	2	0	12	.115	3
522			min	-17.454	1	-74.001	3	-13.133	1	0	3	-.025	1	-.238	2
523		15	max	-.173	10	57.033	2	1.599	9	0	2	0	12	.14	3
524			min	-17.454	1	-25.963	3	-1.115	2	0	3	-.028	1	-.292	2
525		16	max	-.173	10	22.075	3	15.991	1	0	2	0	3	.141	3
526			min	-17.454	1	-44.419	2	.158	10	0	3	-.023	1	-.295	2
527		17	max	-.173	10	70.114	3	30.553	1	0	2	.001	3	.118	3
528			min	-17.454	1	-145.871	2	.96	15	0	3	-.012	1	-.247	2
529		18	max	-.173	10	118.152	3	45.116	1	0	2	.007	1	.071	3
530			min	-17.454	1	-247.323	2	1.402	15	0	3	-.001	10	-.149	2
531		19	max	-.173	10	166.19	3	59.678	1	0	2	.033	1	0	2
532			min	-17.454	1	-348.775	2	1.845	15	0	3	0	10	0	3
533	M15	1	max	0	1	.923	3	.107	3	0	1	0	1	0	1
534			min	-59.178	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.821	3	.107	3	0	1	0	1	0	1
536			min	-59.232	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.718	3	.107	3	0	1	0	1	0	1
538			min	-59.286	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.616	3	.107	3	0	1	0	1	0	1
540			min	-59.34	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.513	3	.107	3	0	1	0	1	0	1
542			min	-59.394	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.41	3	.107	3	0	1	0	1	0	1
544			min	-59.448	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.308	3	.107	3	0	1	0	3	0	1
546			min	-59.502	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.205	3	.107	3	0	1	0	3	0	1
548			min	-59.556	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.103	3	.107	3	0	1	0	3	0	1
550			min	-59.61	3	0	1	0	1	0	3	0	1	-.001	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.107	3	0	1	0	3	0	1
552		min	-59.664	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.107	3	0	1	0	3	0	1
554		min	-59.718	3	-.103	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.107	3	0	1	0	3	0	1
556		min	-59.772	3	-.205	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.107	3	0	1	0	3	0	1
558		min	-59.826	3	-.308	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.107	3	0	1	0	3	0	1
560		min	-59.88	3	-.41	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.107	3	0	1	0	3	0	1
562		min	-59.934	3	-.513	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.107	3	0	1	0	3	0	1
564		min	-59.988	3	-.616	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.107	3	0	1	0	3	0	1
566		min	-60.042	3	-.718	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.107	3	0	1	0	3	0	1
568		min	-60.095	3	-.821	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.107	3	0	1	0	3	0	1
570		min	-60.149	3	-.923	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.58	4	.04	1	0	3	0	3	0	1
572		min	-59.055	3	0	2	-.041	3	0	1	0	1	0	1
573	2	max	0	2	1.404	4	.04	1	0	3	0	3	0	2
574		min	-59.001	3	0	2	-.041	3	0	1	0	1	0	4
575	3	max	0	2	1.229	4	.04	1	0	3	0	3	0	2
576		min	-58.947	3	0	2	-.041	3	0	1	0	1	0	4
577	4	max	0	2	1.053	4	.04	1	0	3	0	3	0	2
578		min	-58.893	3	0	2	-.041	3	0	1	0	1	-.001	4
579	5	max	0	2	.878	4	.04	1	0	3	0	3	0	2
580		min	-58.839	3	0	2	-.041	3	0	1	0	1	-.001	4
581	6	max	0	2	.702	4	.04	1	0	3	0	3	0	2
582		min	-58.785	3	0	2	-.041	3	0	1	0	1	-.002	4
583	7	max	0	2	.527	4	.04	1	0	3	0	3	0	2
584		min	-58.731	3	0	2	-.041	3	0	1	0	1	-.002	4
585	8	max	0	2	.351	4	.04	1	0	3	0	3	0	2
586		min	-58.677	3	0	2	-.041	3	0	1	0	1	-.002	4
587	9	max	0	2	.176	4	.04	1	0	3	0	3	0	2
588		min	-58.623	3	0	2	-.041	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.04	1	0	3	0	3	0	2
590		min	-58.569	3	0	1	-.041	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.04	1	0	3	0	3	0	2
592		min	-58.515	3	-.176	4	-.041	3	0	1	0	1	-.002	4
593	12	max	.048	13	0	2	.04	1	0	3	0	3	0	2
594		min	-58.461	3	-.351	4	-.041	3	0	1	0	1	-.002	4
595	13	max	.122	13	0	2	.04	1	0	3	0	1	0	2
596		min	-58.407	3	-.527	4	-.041	3	0	1	0	4	-.002	4
597	14	max	.197	13	0	2	.04	1	0	3	0	1	0	2
598		min	-58.353	3	-.702	4	-.041	3	0	1	0	3	-.002	4
599	15	max	.271	13	0	2	.04	1	0	3	0	1	0	2
600		min	-58.3	3	-.878	4	-.041	3	0	1	0	3	-.001	4
601	16	max	.345	13	0	2	.04	1	0	3	0	1	0	2
602		min	-58.246	3	-1.053	4	-.041	3	0	1	0	3	-.001	4
603	17	max	.419	13	0	2	.04	1	0	3	0	1	0	2
604		min	-58.192	3	-1.229	4	-.041	3	0	1	0	3	0	4
605	18	max	.494	13	0	2	.04	1	0	3	0	1	0	2
606		min	-58.138	3	-1.404	4	-.041	3	0	1	0	3	0	4
607	19	max	.585	4	0	2	.04	1	0	3	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-58.084	3	-1.58	4	-.041	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.005	2	.003	1	-3.163e-6	10	NC	3	NC	1	
2			min	-.003	3	-.004	3	-.001	3	-2.415e-4	1	5537.415	2	NC	1	
3			2	max	.002	1	.005	2	.003	1	-3.036e-6	10	NC	3	NC	1
4				min	-.003	3	-.004	3	-.001	3	-2.317e-4	1	6016.825	2	NC	1
5			3	max	.002	1	.005	2	.002	1	-2.909e-6	10	NC	1	NC	1
6				min	-.002	3	-.004	3	-.001	3	-2.218e-4	1	6582.598	2	NC	1
7			4	max	.002	1	.004	2	.002	1	-2.782e-6	10	NC	1	NC	1
8				min	-.002	3	-.004	3	0	3	-2.12e-4	1	7255.117	2	NC	1
9			5	max	.002	1	.004	2	.002	1	-2.655e-6	10	NC	1	NC	1
10				min	-.002	3	-.004	3	0	3	-2.021e-4	1	8061.424	2	NC	1
11			6	max	.001	1	.003	2	.002	1	-2.527e-6	10	NC	1	NC	1
12				min	-.002	3	-.004	3	0	3	-1.922e-4	1	9037.989	2	NC	1
13			7	max	.001	1	.003	2	.002	1	-2.4e-6	10	NC	1	NC	1
14				min	-.002	3	-.003	3	0	3	-1.824e-4	1	NC	1	NC	1
15			8	max	.001	1	.003	2	.001	1	-2.273e-6	10	NC	1	NC	1
16				min	-.002	3	-.003	3	0	3	-1.725e-4	1	NC	1	NC	1
17			9	max	.001	1	.002	2	.001	1	-2.146e-6	10	NC	1	NC	1
18				min	-.002	3	-.003	3	0	3	-1.627e-4	1	NC	1	NC	1
19			10	max	.001	1	.002	2	.001	1	-2.018e-6	10	NC	1	NC	1
20				min	-.001	3	-.003	3	0	3	-1.528e-4	1	NC	1	NC	1
21		11	max	0	1	.002	2	0	1	-1.891e-6	10	NC	1	NC	1	
22			min	-.001	3	-.003	3	0	3	-1.43e-4	1	NC	1	NC	1	
23		12	max	0	1	.001	2	0	1	-1.764e-6	10	NC	1	NC	1	
24			min	-.001	3	-.002	3	0	3	-1.331e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	-1.637e-6	10	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-1.233e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	0	1	-1.509e-6	10	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-1.134e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-1.382e-6	10	NC	1	NC	1	
30			min	0	3	-.001	3	0	3	-1.035e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.255e-6	10	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-9.369e-5	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.128e-6	10	NC	1	NC	1	
34			min	0	3	0	3	0	3	-8.384e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.001e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-7.398e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-8.733e-7	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-6.412e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.918e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.973e-7	10	NC	1	NC	1	
41			2	max	0	9	0	2	0	10	3.864e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	4.807e-7	10	NC	1	NC	1
43			3	max	0	9	0	2	0	10	4.81e-5	1	NC	1	NC	1
44				min	0	2	-.001	3	0	1	5.642e-7	10	NC	1	NC	1
45			4	max	0	9	0	2	0	12	5.756e-5	1	NC	1	NC	1
46				min	0	2	-.002	3	0	1	6.476e-7	10	NC	1	NC	1
47			5	max	0	9	0	2	0	3	6.701e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	7.31e-7	10	NC	1	NC	1
49			6	max	0	9	0	2	0	3	7.647e-5	1	NC	1	NC	1
50				min	0	2	-.003	3	0	1	8.145e-7	10	NC	1	NC	1
51			7	max	0	9	0	2	0	3	8.593e-5	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.004	3	0	9	8.979e-7	10	NC	1	NC	1
53		8	max	0	9	0	2	0	3	9.539e-5	1	NC	1	NC	1
54			min	0	2	-.004	3	0	9	9.813e-7	10	NC	1	NC	1
55		9	max	0	9	0	2	0	3	1.048e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	1.065e-6	10	NC	1	NC	1
57		10	max	0	9	.001	2	0	1	1.143e-4	1	NC	1	NC	1
58			min	0	2	-.005	3	0	10	1.148e-6	10	NC	1	NC	1
59		11	max	0	9	.002	2	0	1	1.238e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	1.232e-6	10	NC	1	NC	1
61		12	max	0	9	.002	2	0	1	1.332e-4	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	1.315e-6	10	NC	1	NC	1
63		13	max	0	9	.003	2	0	1	1.427e-4	1	NC	1	NC	1
64			min	0	2	-.006	3	0	10	1.399e-6	10	NC	1	NC	1
65		14	max	0	9	.004	2	0	1	1.521e-4	1	NC	1	NC	1
66			min	0	2	-.006	3	0	10	1.482e-6	10	NC	1	NC	1
67		15	max	0	9	.004	2	.001	1	1.616e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	1.565e-6	10	NC	1	NC	1
69		16	max	0	9	.005	2	.001	1	1.71e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	1.649e-6	10	8975.15	2	NC	1
71		17	max	0	9	.006	2	.002	1	1.805e-4	1	NC	3	NC	1
72			min	0	2	-.007	3	0	10	1.732e-6	10	7639.53	2	NC	1
73		18	max	0	9	.007	2	.002	1	1.9e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	10	1.816e-6	10	6611.666	2	NC	1
75		19	max	0	9	.008	2	.002	1	1.994e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	10	1.899e-6	10	5811.84	2	NC	1
77	M4	1	max	.002	1	.006	2	0	10	-1.654e-6	10	NC	1	NC	1
78			min	0	3	-.005	3	-.001	1	-2.145e-4	1	NC	1	NC	1
79		2	max	.002	1	.006	2	0	10	-1.654e-6	10	NC	1	NC	1
80			min	0	3	-.005	3	-.001	1	-2.145e-4	1	NC	1	NC	1
81		3	max	.002	1	.006	2	0	10	-1.654e-6	10	NC	1	NC	1
82			min	0	3	-.004	3	-.001	1	-2.145e-4	1	NC	1	NC	1
83		4	max	.001	1	.005	2	0	10	-1.654e-6	10	NC	1	NC	1
84			min	0	3	-.004	3	-.001	1	-2.145e-4	1	NC	1	NC	1
85		5	max	.001	1	.005	2	0	10	-1.654e-6	10	NC	1	NC	1
86			min	0	3	-.004	3	0	1	-2.145e-4	1	NC	1	NC	1
87		6	max	.001	1	.005	2	0	10	-1.654e-6	10	NC	1	NC	1
88			min	0	3	-.004	3	0	1	-2.145e-4	1	NC	1	NC	1
89		7	max	.001	1	.004	2	0	10	-1.654e-6	10	NC	1	NC	1
90			min	0	3	-.003	3	0	1	-2.145e-4	1	NC	1	NC	1
91		8	max	.001	1	.004	2	0	10	-1.654e-6	10	NC	1	NC	1
92			min	0	3	-.003	3	0	1	-2.145e-4	1	NC	1	NC	1
93		9	max	0	1	.003	2	0	10	-1.654e-6	10	NC	1	NC	1
94			min	0	3	-.003	3	0	1	-2.145e-4	1	NC	1	NC	1
95		10	max	0	1	.003	2	0	10	-1.654e-6	10	NC	1	NC	1
96			min	0	3	-.002	3	0	1	-2.145e-4	1	NC	1	NC	1
97		11	max	0	1	.003	2	0	10	-1.654e-6	10	NC	1	NC	1
98			min	0	3	-.002	3	0	1	-2.145e-4	1	NC	1	NC	1
99		12	max	0	1	.002	2	0	10	-1.654e-6	10	NC	1	NC	1
100			min	0	3	-.002	3	0	1	-2.145e-4	1	NC	1	NC	1
101		13	max	0	1	.002	2	0	10	-1.654e-6	10	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-2.145e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	10	-1.654e-6	10	NC	1	NC	1
104			min	0	3	-.001	3	0	1	-2.145e-4	1	NC	1	NC	1
105		15	max	0	1	.001	2	0	10	-1.654e-6	10	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-2.145e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	10	-1.654e-6	10	NC	1	NC	1
108			min	0	3	0	3	0	1	-2.145e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	10	-1.654e-6	10	NC	1	NC	1
110			min	0	3	0	3	0	1	-2.145e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	-1.654e-6	10	NC	1	NC	1
112			min	0	3	0	3	0	1	-2.145e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.654e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-2.145e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.018	2	0	1	2.666e-4	3	NC	3	NC	1
116			min	-.009	3	-.014	3	-.004	3	-8.516e-8	2	1651.802	2	8123.232	3
117		2	max	.006	1	.017	2	0	1	2.607e-4	3	NC	3	NC	1
118			min	-.008	3	-.013	3	-.003	3	-8.065e-8	2	1763.365	2	8703.294	3
119		3	max	.006	1	.016	2	0	1	2.548e-4	3	NC	3	NC	1
120			min	-.008	3	-.012	3	-.003	3	-7.614e-8	2	1890.696	2	9382.546	3
121		4	max	.005	1	.015	2	0	1	2.49e-4	3	NC	3	NC	1
122			min	-.007	3	-.012	3	-.003	3	-9.523e-7	11	2036.956	2	NC	1
123		5	max	.005	1	.014	2	0	1	2.431e-4	3	NC	3	NC	1
124			min	-.007	3	-.011	3	-.003	3	-1.978e-6	11	2206.217	2	NC	1
125		6	max	.005	1	.013	2	0	1	2.372e-4	3	NC	3	NC	1
126			min	-.006	3	-.01	3	-.002	3	-3.12e-6	1	2403.809	2	NC	1
127		7	max	.004	1	.011	2	0	1	2.313e-4	3	NC	3	NC	1
128			min	-.006	3	-.01	3	-.002	3	-5.463e-6	1	2636.846	2	NC	1
129		8	max	.004	1	.01	2	0	1	2.254e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.002	3	-7.806e-6	1	2915.035	2	NC	1
131		9	max	.004	1	.009	2	0	1	2.195e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	-.002	3	-1.015e-5	1	3251.973	2	NC	1
133		10	max	.003	1	.008	2	0	1	2.136e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	-.001	3	-1.249e-5	1	3667.305	2	NC	1
135		11	max	.003	1	.007	2	0	1	2.077e-4	3	NC	3	NC	1
136			min	-.004	3	-.007	3	-.001	3	-1.483e-5	1	4190.505	2	NC	1
137		12	max	.003	1	.006	2	0	1	2.018e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	-.001	3	-1.718e-5	1	4867.898	2	NC	1
139		13	max	.002	1	.005	2	0	1	1.959e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-1.952e-5	1	5776.694	2	NC	1
141		14	max	.002	1	.004	2	0	1	1.9e-4	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-2.186e-5	1	7055.874	2	NC	1
143		15	max	.001	1	.003	2	0	1	1.841e-4	3	NC	1	NC	1
144			min	-.002	3	-.004	3	0	3	-2.421e-5	1	8983.406	2	NC	1
145		16	max	.001	1	.002	2	0	1	1.782e-4	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-2.655e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.723e-4	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-2.889e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.664e-4	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-3.123e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.605e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.358e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.521e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-7.268e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	1.391e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	1	-5.667e-5	3	NC	1	NC	1
157		3	max	0	9	.002	2	0	3	1.262e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-4.066e-5	3	NC	1	NC	1
159		4	max	0	9	.003	2	.001	3	1.133e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-2.464e-5	3	NC	1	NC	1
161		5	max	0	9	.004	2	.001	3	1.004e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-8.633e-6	3	NC	1	NC	1
163		6	max	0	9	.006	2	.002	3	8.742e-6	1	NC	1	NC	1
164			min	0	2	-.007	3	0	1	0	2	8323.139	2	NC	1
165		7	max	0	9	.007	2	.002	3	2.339e-5	3	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166		min	0	2	-.009	3	0	1	0	2	6887.693	2	NC	1
167	8	max	0	9	.008	2	.002	3	3.94e-5	3	NC	3	NC	1
168		min	0	2	-.01	3	0	1	-3.49e-8	13	5832.223	2	NC	1
169	9	max	0	9	.009	2	.002	3	5.541e-5	3	NC	3	NC	1
170		min	0	2	-.011	3	0	1	-3.23e-7	9	5019.427	2	NC	1
171	10	max	0	9	.011	2	.002	3	7.143e-5	3	NC	3	NC	1
172		min	-.001	2	-.012	3	0	1	-1.579e-6	9	4373.107	2	NC	1
173	11	max	0	9	.012	2	.002	3	8.744e-5	3	NC	3	NC	1
174		min	-.001	2	-.013	3	0	1	-2.836e-6	9	3847.352	2	NC	1
175	12	max	0	9	.013	2	.002	3	1.035e-4	3	NC	3	NC	1
176		min	-.001	2	-.015	3	0	1	-4.092e-6	9	3412.604	2	NC	1
177	13	max	0	9	.015	2	.002	3	1.195e-4	3	NC	3	NC	1
178		min	-.001	2	-.016	3	0	1	-5.349e-6	9	3048.764	2	NC	1
179	14	max	0	9	.017	2	.002	3	1.355e-4	3	NC	3	NC	1
180		min	-.002	2	-.016	3	0	1	-6.605e-6	9	2741.534	2	NC	1
181	15	max	0	9	.019	2	.002	3	1.515e-4	3	NC	3	NC	1
182		min	-.002	2	-.017	3	0	1	-7.862e-6	9	2480.356	2	NC	1
183	16	max	0	9	.02	2	.002	3	1.675e-4	3	NC	3	NC	1
184		min	-.002	2	-.018	3	0	1	-9.118e-6	9	2257.2	2	NC	1
185	17	max	.001	9	.022	2	.002	3	1.835e-4	3	NC	3	NC	1
186		min	-.002	2	-.019	3	0	1	-1.037e-5	9	2065.818	2	NC	1
187	18	max	.001	9	.024	2	.002	3	1.995e-4	3	NC	3	NC	1
188		min	-.002	2	-.02	3	0	1	-1.163e-5	9	1901.267	2	NC	1
189	19	max	.001	9	.026	2	.002	3	2.155e-4	3	NC	3	NC	1
190		min	-.002	2	-.02	3	0	1	-1.289e-5	9	1759.582	2	NC	1
191	M8	1	max	.005	1	.021	2	0	-7.24e-8	10	NC	1	NC	1
192		min	-.002	3	-.015	3	-.001	3	-1.715e-4	3	NC	1	NC	1
193	2	max	.005	1	.02	2	0	1	-7.24e-8	10	NC	1	NC	1
194		min	-.002	3	-.014	3	-.001	3	-1.715e-4	3	NC	1	NC	1
195	3	max	.005	1	.019	2	0	1	-7.24e-8	10	NC	1	NC	1
196		min	-.002	3	-.013	3	-.001	3	-1.715e-4	3	NC	1	NC	1
197	4	max	.005	1	.017	2	0	1	-7.24e-8	10	NC	1	NC	1
198		min	-.002	3	-.013	3	0	3	-1.715e-4	3	NC	1	NC	1
199	5	max	.004	1	.016	2	0	1	-7.24e-8	10	NC	1	NC	1
200		min	-.002	3	-.012	3	0	3	-1.715e-4	3	NC	1	NC	1
201	6	max	.004	1	.015	2	0	1	-7.24e-8	10	NC	1	NC	1
202		min	-.001	3	-.011	3	0	3	-1.715e-4	3	NC	1	NC	1
203	7	max	.004	1	.014	2	0	1	-7.24e-8	10	NC	1	NC	1
204		min	-.001	3	-.01	3	0	3	-1.715e-4	3	NC	1	NC	1
205	8	max	.003	1	.013	2	0	1	-7.24e-8	10	NC	1	NC	1
206		min	-.001	3	-.009	3	0	3	-1.715e-4	3	NC	1	NC	1
207	9	max	.003	1	.012	2	0	1	-7.24e-8	10	NC	1	NC	1
208		min	-.001	3	-.008	3	0	3	-1.715e-4	3	NC	1	NC	1
209	10	max	.003	1	.01	2	0	1	-7.24e-8	10	NC	1	NC	1
210		min	0	3	-.008	3	0	3	-1.715e-4	3	NC	1	NC	1
211	11	max	.002	1	.009	2	0	1	-7.24e-8	10	NC	1	NC	1
212		min	0	3	-.007	3	0	3	-1.715e-4	3	NC	1	NC	1
213	12	max	.002	1	.008	2	0	1	-7.24e-8	10	NC	1	NC	1
214		min	0	3	-.006	3	0	3	-1.715e-4	3	NC	1	NC	1
215	13	max	.002	1	.007	2	0	1	-7.24e-8	10	NC	1	NC	1
216		min	0	3	-.005	3	0	3	-1.715e-4	3	NC	1	NC	1
217	14	max	.002	1	.006	2	0	1	-7.24e-8	10	NC	1	NC	1
218		min	0	3	-.004	3	0	3	-1.715e-4	3	NC	1	NC	1
219	15	max	.001	1	.005	2	0	1	-7.24e-8	10	NC	1	NC	1
220		min	0	3	-.003	3	0	3	-1.715e-4	3	NC	1	NC	1
221	16	max	0	1	.003	2	0	1	-7.24e-8	10	NC	1	NC	1
222		min	0	3	-.003	3	0	3	-1.715e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.002	2	0	1	-7.24e-8	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.715e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-7.24e-8	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.715e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-7.24e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.715e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.005	2	0	3	2.571e-4	1	NC	3	NC	1
230			min	-.002	3	-.005	3	-.001	1	-3.58e-4	3	5548.461	2	NC	1
231		2	max	.002	1	.005	2	0	3	2.446e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.001	1	-3.479e-4	3	6029.068	2	NC	1
233		3	max	.002	1	.005	2	0	3	2.321e-4	1	NC	1	NC	1
234			min	-.002	3	-.004	3	0	1	-3.378e-4	3	6596.296	2	NC	1
235		4	max	.002	1	.004	2	0	3	2.197e-4	1	NC	1	NC	1
236			min	-.002	3	-.004	3	0	1	-3.277e-4	3	7270.602	2	NC	1
237		5	max	.002	1	.004	2	0	3	2.072e-4	1	NC	1	NC	1
238			min	-.002	3	-.004	3	0	1	-3.176e-4	3	8079.122	2	NC	1
239		6	max	.001	1	.003	2	0	3	1.947e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	0	1	-3.075e-4	3	9058.464	2	NC	1
241		7	max	.001	1	.003	2	0	3	1.823e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	0	1	-2.974e-4	3	NC	1	NC	1
243		8	max	.001	1	.003	2	0	3	1.698e-4	1	NC	1	NC	1
244			min	-.002	3	-.003	3	0	1	-2.873e-4	3	NC	1	NC	1
245		9	max	.001	1	.002	2	0	3	1.573e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-2.772e-4	3	NC	1	NC	1
247		10	max	.001	1	.002	2	0	3	1.449e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-2.671e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	1.324e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-2.571e-4	3	NC	1	NC	1
251		12	max	0	1	.001	2	0	3	1.2e-4	1	NC	1	NC	1
252			min	0	3	-.002	3	0	1	-2.47e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	1.075e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-2.369e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	9.502e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-2.268e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	8.256e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.167e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	7.01e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-2.066e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	5.763e-5	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-1.965e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	4.517e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.864e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.271e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.763e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	8.048e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.512e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	1	6.456e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.751e-5	1	NC	1	NC	1
271		3	max	0	9	0	2	0	1	4.864e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-3.991e-5	1	NC	1	NC	1
273		4	max	0	9	0	2	0	2	3.272e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.23e-5	1	NC	1	NC	1
275		5	max	0	9	0	2	0	2	1.68e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-6.469e-5	1	NC	1	NC	1
277		6	max	0	9	0	2	0	2	8.768e-7	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-7.709e-5	1	NC	1	NC	1
279		7	max	0	9	0	2	0	10	-8.533e-7	10	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	2.177e-4	3	NC	1	NC	1
338			min	0	3	0	3	0	10	1.509e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.177e-4	3	NC	1	NC	1
340			min	0	3	0	3	0	10	1.509e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.177e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	1.509e-6	10	NC	1	NC	1
343	M1	1	max	.005	3	.021	3	.002	3	8.376e-3	1	NC	1	NC	1
344			min	-.005	2	-.019	1	0	1	-1.034e-2	3	NC	1	NC	1
345		2	max	.005	3	.011	3	.002	3	4.085e-3	1	NC	4	NC	1
346			min	-.005	2	-.01	1	-.002	1	-5.09e-3	3	4999.456	3	NC	1
347		3	max	.005	3	.002	3	.001	3	6.502e-5	3	NC	4	NC	1
348			min	-.005	2	-.002	1	-.003	1	-1.259e-4	1	2596.174	3	NC	1
349		4	max	.005	3	.006	2	0	3	6.316e-5	3	NC	4	NC	1
350			min	-.006	2	-.005	3	-.003	1	-1.017e-4	1	1857.912	2	NC	1
351		5	max	.005	3	.012	2	0	3	6.13e-5	3	NC	4	NC	1
352			min	-.006	2	-.011	3	-.003	1	-7.741e-5	1	1476.081	2	NC	1
353		6	max	.005	3	.017	2	0	3	5.943e-5	3	NC	5	NC	1
354			min	-.006	2	-.016	3	-.003	1	-5.316e-5	1	1258.491	2	NC	1
355		7	max	.005	3	.021	2	0	3	5.757e-5	3	NC	5	NC	1
356			min	-.006	2	-.02	3	-.003	1	-2.916e-5	9	1124.869	2	NC	1
357		8	max	.005	3	.024	2	0	3	5.571e-5	3	NC	5	NC	1
358			min	-.006	2	-.022	3	-.002	1	-1.195e-5	9	1041.714	2	NC	1
359		9	max	.005	3	.026	2	0	3	5.385e-5	3	NC	5	NC	1
360			min	-.006	2	-.023	3	-.002	1	4.216e-7	15	993.246	2	NC	1
361		10	max	.005	3	.027	2	0	3	5.199e-5	3	NC	5	NC	1
362			min	-.006	2	-.024	3	0	1	9.576e-7	10	971.99	2	NC	1
363		11	max	.005	3	.027	2	0	3	6.807e-5	1	NC	5	NC	1
364			min	-.006	2	-.023	3	0	9	1.124e-6	10	975.335	2	NC	1
365		12	max	.005	3	.025	2	0	1	9.232e-5	1	NC	5	NC	1
366			min	-.006	2	-.021	3	0	10	1.291e-6	10	1004.535	2	NC	1
367		13	max	.005	3	.022	2	.001	1	1.166e-4	1	NC	5	NC	1
368			min	-.006	2	-.018	3	0	10	1.458e-6	10	1065.34	2	NC	1
369		14	max	.005	3	.018	2	.002	1	1.408e-4	1	NC	5	NC	1
370			min	-.006	2	-.014	3	0	10	1.625e-6	10	1170.847	2	NC	1
371		15	max	.005	3	.012	2	.002	1	1.651e-4	1	NC	4	NC	1
372			min	-.006	2	-.01	3	0	10	1.792e-6	10	1349.589	2	NC	1
373		16	max	.005	3	.005	2	.002	1	1.832e-4	1	NC	4	NC	1
374			min	-.006	2	-.004	3	0	10	1.935e-6	10	1670.603	2	NC	1
375		17	max	.005	3	.002	3	.001	1	5.682e-5	1	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	1.513e-6	10	2348.385	2	NC	1
377		18	max	.005	3	.009	3	0	1	5.271e-3	2	NC	4	NC	1
378			min	-.006	2	-.013	2	0	10	-2.595e-3	3	4536.925	2	NC	1
379		19	max	.005	3	.016	3	0	3	1.061e-2	2	NC	1	NC	1
380			min	-.006	2	-.024	2	0	1	-5.276e-3	3	NC	1	NC	1
381	M5	1	max	.014	3	.067	3	.002	3	2.42e-6	3	NC	1	NC	1
382			min	-.018	2	-.063	1	0	1	0	1	NC	1	NC	1
383		2	max	.014	3	.036	3	.003	3	7.016e-5	3	NC	4	NC	1
384			min	-.018	2	-.033	1	0	1	-1.952e-5	1	1561.503	3	NC	1
385		3	max	.014	3	.008	3	.004	3	1.366e-4	3	NC	5	NC	1
386			min	-.018	2	-.005	1	0	1	-3.865e-5	1	805.689	1	NC	1
387		4	max	.014	3	.019	2	.004	3	1.349e-4	3	NC	5	NC	1
388			min	-.018	2	-.016	3	0	1	-3.644e-5	9	568.137	1	NC	1
389		5	max	.014	3	.039	2	.005	3	1.331e-4	3	NC	5	NC	1
390			min	-.018	2	-.036	3	0	1	-3.425e-5	9	453.8	1	NC	1
391		6	max	.014	3	.056	2	.005	3	1.313e-4	3	NC	5	NC	1
392			min	-.018	2	-.051	3	0	1	-3.207e-5	9	387.21	2	NC	1
393		7	max	.014	3	.07	2	.005	3	1.295e-4	3	NC	5	NC	1



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 15° 120mph 30psf 4.5ft 7-05 N3med39



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	10	-1.624e-6	10	NC	4	NC	1
452			min	-.006	2	-.003	2	-.003	1	-1.343e-4	1	2348.955	2	NC	1
453		18	max	.005	3	.009	3	0	10	2.604e-3	3	NC	4	NC	1
454			min	-.006	2	-.013	2	-.002	1	-5.271e-3	2	4537.988	2	NC	1
455		19	max	.005	3	.016	3	0	3	5.275e-3	3	NC	1	NC	1
456			min	-.006	2	-.024	2	0	1	-1.061e-2	2	NC	1	NC	1
457	M13	1	max	.001	1	.021	3	.005	3	3.648e-3	3	NC	1	NC	1
458			min	-.002	3	-.019	1	-.005	2	-3.412e-3	1	NC	1	NC	1
459		2	max	.001	1	.081	3	.003	3	4.544e-3	3	NC	4	NC	1
460			min	-.002	3	-.068	1	-.004	2	-4.275e-3	1	1800.121	3	NC	1
461		3	max	0	1	.131	3	.008	1	5.44e-3	3	NC	5	NC	2
462			min	-.002	3	-.108	1	-.003	10	-5.138e-3	1	983.062	3	9103.237	1
463		4	max	0	1	.164	3	.013	1	6.336e-3	3	NC	5	NC	2
464			min	-.002	3	-.135	1	-.004	10	-6.001e-3	1	756.704	3	6475.212	1
465		5	max	0	1	.176	3	.015	1	7.232e-3	3	NC	5	NC	2
466			min	-.002	3	-.146	1	-.005	10	-6.863e-3	1	695.544	3	5966.302	1
467		6	max	0	1	.169	3	.012	1	8.128e-3	3	NC	5	NC	2
468			min	-.002	3	-.141	1	-.006	10	-7.726e-3	1	730.931	3	6970.848	1
469		7	max	0	1	.145	3	.008	3	9.024e-3	3	NC	5	NC	1
470			min	-.002	3	-.123	1	-.009	2	-8.589e-3	1	871.13	3	NC	1
471		8	max	0	1	.112	3	.01	3	9.92e-3	3	NC	5	NC	1
472			min	-.002	3	-.098	1	-.013	2	-9.452e-3	1	1182.947	3	NC	1
473		9	max	0	1	.082	3	.012	3	1.082e-2	3	NC	4	NC	1
474			min	-.002	3	-.074	1	-.017	2	-1.031e-2	1	1781.016	3	9599.326	2
475		10	max	0	1	.067	3	.014	3	1.171e-2	3	NC	4	NC	1
476			min	-.002	3	-.063	1	-.018	2	-1.118e-2	1	2323.429	3	8375.474	2
477		11	max	0	1	.082	3	.015	3	1.082e-2	3	NC	4	NC	1
478			min	-.002	3	-.074	1	-.017	2	-1.032e-2	1	1781.015	3	9599.382	2
479		12	max	0	1	.112	3	.016	3	9.922e-3	3	NC	5	NC	1
480			min	-.002	3	-.098	1	-.013	2	-9.453e-3	1	1182.947	3	9618.845	3
481		13	max	0	1	.145	3	.015	3	9.026e-3	3	NC	5	NC	1
482			min	-.002	3	-.123	1	-.009	2	-8.59e-3	1	871.13	3	9971.571	3
483		14	max	0	1	.169	3	.014	3	8.131e-3	3	NC	5	NC	2
484			min	-.002	3	-.141	1	-.006	10	-7.727e-3	1	730.931	3	6963.382	1
485		15	max	0	1	.176	3	.015	1	7.236e-3	3	NC	5	NC	2
486			min	-.002	3	-.146	1	-.005	10	-6.865e-3	1	695.544	3	5969.945	1
487		16	max	0	1	.164	3	.013	1	6.341e-3	3	NC	5	NC	2
488			min	-.002	3	-.135	1	-.004	10	-6.002e-3	1	756.704	3	6488.764	1
489		17	max	0	1	.131	3	.008	1	5.446e-3	3	NC	5	NC	2
490			min	-.002	3	-.108	1	-.003	10	-5.139e-3	1	983.062	3	9139.022	1
491		18	max	0	1	.081	3	.006	3	4.551e-3	3	NC	4	NC	1
492			min	-.002	3	-.068	1	-.004	2	-4.277e-3	1	1800.122	3	NC	1
493		19	max	0	1	.021	3	.005	3	3.656e-3	3	NC	1	NC	1
494			min	-.002	3	-.019	1	-.005	2	-3.414e-3	1	NC	1	NC	1
495	M16	1	max	0	1	.016	3	.005	3	4.016e-3	2	NC	1	NC	1
496			min	0	3	-.024	2	-.006	2	-2.742e-3	3	NC	1	NC	1
497		2	max	0	1	.046	3	.006	3	5.021e-3	2	NC	4	NC	1
498			min	0	3	-.085	2	-.004	2	-3.4e-3	3	1753.56	2	NC	1
499		3	max	0	1	.072	3	.009	3	6.026e-3	2	NC	5	NC	2
500			min	0	3	-.136	2	-.003	10	-4.057e-3	3	956.385	2	9184.374	1
501		4	max	0	1	.09	3	.013	1	7.031e-3	2	NC	5	NC	2
502			min	0	3	-.171	2	-.004	10	-4.715e-3	3	734.455	2	6539.728	1
503		5	max	0	1	.097	3	.014	1	8.036e-3	2	NC	5	NC	2
504			min	0	3	-.184	2	-.005	10	-5.372e-3	3	672.529	2	6041.358	1
505		6	max	0	1	.095	3	.014	3	9.042e-3	2	NC	5	NC	2
506			min	0	3	-.177	2	-.006	10	-6.03e-3	3	702.354	2	7100.384	1
507		7	max	0	1	.085	3	.015	3	1.005e-2	2	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.154	2	-.009	2	-6.687e-3	3	828.186	2	NC	1
509	8	max	0	1	.071	3	.015	3	1.105e-2	2	NC	5	NC	1
510		min	0	3	-.121	2	-.013	2	-7.345e-3	3	1103.095	2	NC	1
511	9	max	0	1	.056	3	.015	3	1.206e-2	2	NC	4	NC	1
512		min	0	3	-.091	2	-.017	2	-8.003e-3	3	1605.454	2	9393.915	2
513	10	max	0	1	.05	3	.014	3	1.306e-2	2	NC	4	NC	1
514		min	0	3	-.077	2	-.019	2	-8.66e-3	3	2034.259	2	8215.159	2
515	11	max	0	1	.056	3	.013	3	1.206e-2	2	NC	4	NC	1
516		min	0	3	-.091	2	-.017	2	-8.002e-3	3	1605.454	2	9393.954	2
517	12	max	0	1	.07	3	.012	3	1.105e-2	2	NC	5	NC	1
518		min	0	3	-.121	2	-.013	2	-7.344e-3	3	1103.095	2	NC	1
519	13	max	0	1	.085	3	.011	3	1.005e-2	2	NC	5	NC	1
520		min	0	3	-.154	2	-.009	2	-6.685e-3	3	828.186	2	NC	1
521	14	max	0	1	.095	3	.011	1	9.042e-3	2	NC	5	NC	2
522		min	0	3	-.177	2	-.006	10	-6.027e-3	3	702.354	2	7109.92	1
523	15	max	0	1	.097	3	.014	1	8.037e-3	2	NC	5	NC	2
524		min	0	3	-.184	2	-.005	10	-5.369e-3	3	672.529	2	6058.322	1
525	16	max	0	1	.09	3	.013	1	7.032e-3	2	NC	5	NC	2
526		min	0	3	-.171	2	-.004	10	-4.71e-3	3	734.455	2	6568.263	1
527	17	max	0	1	.072	3	.008	1	6.027e-3	2	NC	5	NC	2
528		min	0	3	-.136	2	-.003	10	-4.052e-3	3	956.385	2	9244.564	1
529	18	max	0	1	.046	3	.005	3	5.022e-3	2	NC	4	NC	1
530		min	0	3	-.085	2	-.004	2	-3.394e-3	3	1753.56	2	NC	1
531	19	max	0	1	.016	3	.005	3	4.017e-3	2	NC	1	NC	1
532		min	0	3	-.024	2	-.006	2	-2.736e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.137e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.913e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	7.759e-4	3	NC	1	NC	1
536		min	0	2	-.003	4	0	3	-5.149e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	1	1.238e-3	3	NC	1	NC	1
538		min	0	2	-.006	4	-.003	3	-9.807e-4	2	NC	1	NC	1
539	4	max	0	3	-.002	15	.006	1	1.7e-3	3	NC	3	NC	4
540		min	0	2	-.009	4	-.006	3	-1.447e-3	2	7138.056	4	7047.914	3
541	5	max	0	3	-.003	15	.009	1	2.163e-3	3	NC	5	NC	4
542		min	0	2	-.011	4	-.01	3	-1.923e-3	1	5569.897	4	4586.52	3
543	6	max	0	3	-.003	15	.013	1	2.625e-3	3	NC	5	NC	4
544		min	0	2	-.013	4	-.014	3	-2.4e-3	1	4687.657	4	3320.106	3
545	7	max	0	3	-.004	15	.017	1	3.087e-3	3	NC	5	NC	4
546		min	0	2	-.015	4	-.019	3	-2.878e-3	1	4157.106	4	2584.389	3
547	8	max	0	3	-.004	15	.021	1	3.549e-3	3	NC	5	NC	4
548		min	-.001	2	-.016	4	-.023	3	-3.355e-3	1	3838.696	4	2123.961	3
549	9	max	0	3	-.004	15	.025	1	4.012e-3	3	NC	5	NC	4
550		min	-.001	2	-.017	4	-.027	3	-3.833e-3	1	3667.309	4	1823.513	3
551	10	max	0	3	-.004	15	.028	1	4.474e-3	3	NC	15	NC	4
552		min	-.001	2	-.017	4	-.03	3	-4.31e-3	1	3613.09	4	1625.442	3
553	11	max	0	3	-.004	15	.03	1	4.936e-3	3	NC	5	NC	5
554		min	-.001	2	-.017	4	-.032	3	-4.787e-3	1	3667.309	4	1499.583	3
555	12	max	0	3	-.004	15	.031	1	5.398e-3	3	NC	5	NC	5
556		min	-.002	2	-.017	4	-.033	3	-5.265e-3	1	3838.696	4	1430.914	3
557	13	max	0	3	-.004	15	.031	1	5.86e-3	3	NC	5	NC	5
558		min	-.002	2	-.015	4	-.033	3	-5.742e-3	1	4157.106	4	1414.865	3
559	14	max	0	3	-.003	15	.029	1	6.323e-3	3	NC	5	NC	5
560		min	-.002	2	-.014	4	-.031	3	-6.22e-3	1	4687.657	4	1457.321	3
561	15	max	0	3	-.002	12	.025	1	6.785e-3	3	NC	5	NC	4
562		min	-.002	2	-.012	4	-.027	3	-6.697e-3	1	5569.897	4	1580.676	3
563	16	max	0	3	-.001	12	.019	1	7.247e-3	3	NC	3	NC	4
564		min	-.002	2	-.009	4	-.02	3	-7.175e-3	1	7138.056	4	1846.078	3

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...]	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	0	3	.01	1	7.709e-3	3	NC	1	NC	4
566			min	-.002	2	-.007	4	-.011	3	-7.652e-3	1	NC	1	2445.627	3
567		18	max	0	3	.002	3	.001	9	8.172e-3	3	NC	1	NC	4
568			min	-.003	2	-.004	4	-.005	2	-8.13e-3	1	NC	1	4351.405	3
569		19	max	0	3	.004	3	.016	3	8.634e-3	3	NC	1	NC	1
570			min	-.003	2	-.002	9	-.019	2	-8.607e-3	1	NC	1	NC	1
571	M16A	1	max	0	10	0	3	.005	3	2.558e-3	3	NC	1	NC	1
572			min	0	3	-.001	1	-.006	2	-2.563e-3	2	NC	1	NC	1
573		2	max	0	10	0	15	.001	9	2.447e-3	3	NC	1	NC	1
574			min	0	3	-.003	4	-.001	2	-2.443e-3	2	NC	1	NC	1
575		3	max	0	10	-.001	15	.005	1	2.336e-3	3	NC	1	NC	4
576			min	0	3	-.006	4	-.004	3	-2.323e-3	2	NC	1	6935.617	3
577		4	max	0	10	-.002	15	.008	1	2.225e-3	3	NC	3	NC	4
578			min	0	3	-.009	4	-.008	3	-2.203e-3	2	7138.056	4	5266.081	3
579		5	max	0	10	-.003	15	.009	1	2.113e-3	3	NC	5	NC	4
580			min	0	3	-.011	4	-.01	3	-2.083e-3	2	5569.897	4	4539.148	3
581		6	max	0	10	-.003	15	.011	1	2.002e-3	3	NC	5	NC	4
582			min	0	3	-.013	4	-.011	3	-1.963e-3	2	4687.657	4	4217.064	3
583		7	max	0	10	-.004	15	.011	1	1.891e-3	3	NC	5	NC	4
584			min	0	3	-.015	4	-.012	3	-1.843e-3	2	4157.106	4	4130.73	3
585		8	max	0	10	-.004	15	.011	1	1.78e-3	3	NC	5	NC	4
586			min	0	3	-.016	4	-.012	3	-1.723e-3	2	3838.696	4	4221.382	3
587		9	max	0	10	-.004	15	.01	1	1.669e-3	3	NC	5	NC	4
588			min	0	3	-.017	4	-.011	3	-1.603e-3	2	3667.309	4	4479.267	3
589		10	max	0	10	-.004	15	.009	1	1.558e-3	3	NC	15	NC	4
590			min	0	3	-.017	4	-.01	3	-1.483e-3	2	3613.09	4	4928.835	3
591		11	max	0	10	-.004	15	.008	1	1.447e-3	3	NC	5	NC	4
592			min	0	3	-.017	4	-.009	3	-1.363e-3	2	3667.309	4	5633.326	3
593		12	max	0	10	-.004	15	.007	1	1.335e-3	3	NC	5	NC	4
594			min	0	3	-.016	4	-.007	3	-1.243e-3	2	3838.696	4	6718.269	3
595		13	max	0	10	-.004	15	.005	1	1.224e-3	3	NC	5	NC	2
596			min	0	3	-.015	4	-.006	3	-1.123e-3	2	4157.106	4	8431.611	3
597		14	max	0	10	-.003	15	.004	1	1.113e-3	3	NC	5	NC	1
598			min	0	3	-.013	4	-.004	3	-1.004e-3	2	4687.657	4	NC	



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.