

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	115 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 20.76 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.05	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_s$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	123 in
$\Phi F_{ty \text{ STRONG-AXIS}}$ =	25.07 ksi
$\Phi F_{ty \text{ WEAK-AXIS}}$ =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.050 k-ft
$M_z$ =	0.267 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>97%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty \text{ AXIAL}}$ =	31.09 ksi
$\Phi F_{ty \text{ STRONG-AXIS}}$ =	29.35 ksi
$\Phi F_{ty \text{ WEAK-AXIS}}$ =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.185 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.201 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>92%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

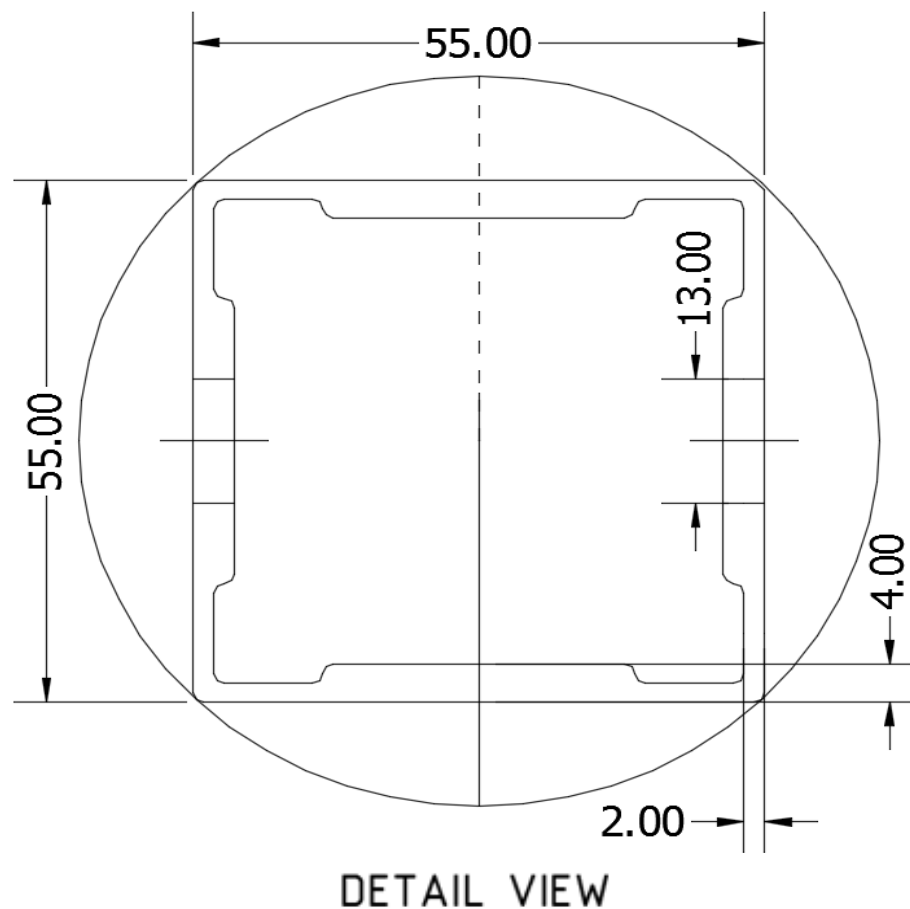
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.480 k-ft
$P_n$ =	0.704 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>37%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

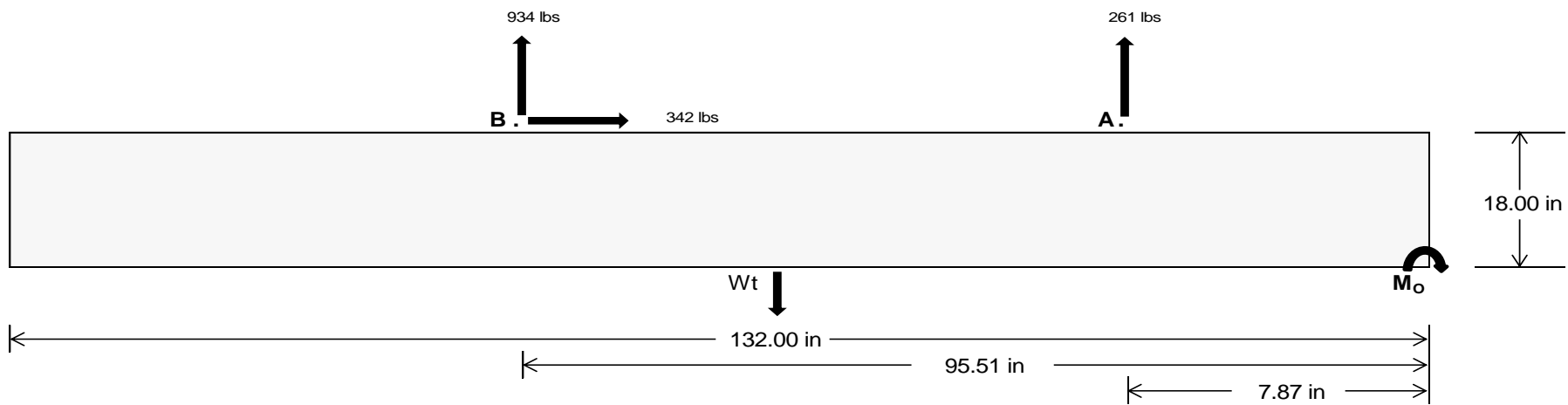
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.022 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>15%</u>



PVMMax 60 Cell 2V 15° 115mph 30psf 10.25ft 7-10.xlsx | Page 5

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 97425.0$  in-lbs  
Resisting Force Required = 1476.14 lbs  
S.F. = 1.67  
Weight Required = 2460.23 lbs  
Minimum Width = 22 in  
Weight Provided = 4386.25 lbs

### Sliding

Force = 342.29 lbs  
Friction = 0.4  
Weight Required = 855.72 lbs  
Resisting Weight = 4386.25 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 342.29 lbs  
Cohesion = 130 psf  
Area = 20.17 ft<sup>2</sup>  
Resisting = 2193.13 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 132in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

	Ballast Width			
	22 in	23 in	24 in	25 in
$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$	4386 lbs	4586 lbs	4785 lbs	4984 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	1767 lbs	1767 lbs	1767 lbs	1767 lbs	1349 lbs	1349 lbs	1349 lbs	1349 lbs	2206 lbs	2206 lbs	2206 lbs	2206 lbs	-523 lbs	-523 lbs	-523 lbs	-523 lbs
$F_B$	1831 lbs	1831 lbs	1831 lbs	1831 lbs	1400 lbs	1400 lbs	1400 lbs	1400 lbs	2288 lbs	2288 lbs	2288 lbs	2288 lbs	-1868 lbs	-1868 lbs	-1868 lbs	-1868 lbs
$F_V$	157 lbs	157 lbs	157 lbs	157 lbs	614 lbs	614 lbs	614 lbs	614 lbs	568 lbs	568 lbs	568 lbs	568 lbs	-685 lbs	-685 lbs	-685 lbs	-685 lbs
$P_{total}$	7985 lbs	8184 lbs	8384 lbs	8583 lbs	7135 lbs	7334 lbs	7534 lbs	7733 lbs	8880 lbs	9079 lbs	9279 lbs	9478 lbs	241 lbs	361 lbs	480 lbs	600 lbs
$M$	4292 lbs-ft	4292 lbs-ft	4292 lbs-ft	4292 lbs-ft	4014 lbs-ft	4014 lbs-ft	4014 lbs-ft	4014 lbs-ft	5911 lbs-ft	5911 lbs-ft	5911 lbs-ft	5911 lbs-ft	1035 lbs-ft	1035 lbs-ft	1035 lbs-ft	1035 lbs-ft
$e$	0.54 ft	0.52 ft	0.51 ft	0.50 ft	0.56 ft	0.55 ft	0.53 ft	0.52 ft	0.67 ft	0.65 ft	0.64 ft	0.62 ft	4.29 ft	2.87 ft	2.15 ft	1.72 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	279.9 psf	277.2 psf	274.7 psf	272.4 psf	245.2 psf	244.0 psf	242.9 psf	241.9 psf	280.4 psf	277.7 psf	275.2 psf	272.9 psf	0.0 psf	0.0 psf	0.0 psf	1.6 psf
$f_{max}$	512.0 psf	499.2 psf	487.5 psf	476.7 psf	462.4 psf	451.7 psf	442.0 psf	433.0 psf	600.2 psf	583.6 psf	568.3 psf	554.3 psf	72.6 psf	47.7 psf	47.8 psf	50.8 psf

Maximum Bearing Pressure = 600 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

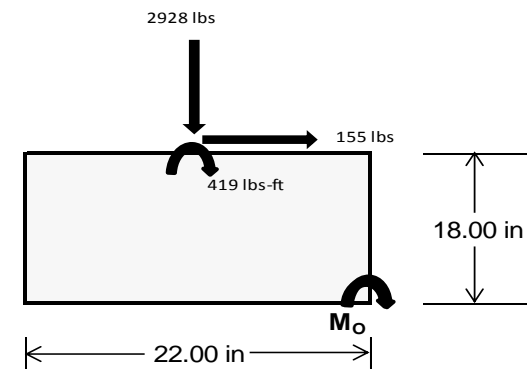
### Overturning Check

$M_O = 2032.2 \text{ ft-lbs}$   
 Resisting Force Required = 2216.99 lbs  
 S.F. = 1.67  
 Weight Required = 3694.98 lbs  
 Minimum Width = 22 in  
 Weight Provided = 4386.25 lbs

*A minimum 132in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_Y$	256 lbs	661 lbs	228 lbs	971 lbs	2928 lbs	949 lbs	85 lbs	193 lbs	57 lbs
$F_V$	216 lbs	213 lbs	218 lbs	161 lbs	155 lbs	169 lbs	216 lbs	214 lbs	217 lbs
$P_{\text{total}}$	5686 lbs	6091 lbs	5658 lbs	6140 lbs	8097 lbs	6118 lbs	1673 lbs	1781 lbs	1644 lbs
$M$	866 lbs-ft	859 lbs-ft	871 lbs-ft	657 lbs-ft	652 lbs-ft	681 lbs-ft	862 lbs-ft	856 lbs-ft	864 lbs-ft
$e$	0.15 ft	0.14 ft	0.15 ft	0.11 ft	0.08 ft	0.11 ft	0.52 ft	0.48 ft	0.53 ft
$L/6$	0.31 ft	0.31 ft	0.31 ft	0.31 ft	0.31 ft	0.31 ft	0.31 ft	0.31 ft	0.31 ft
$f_{\text{min}}$	141.4 psf	162.6 psf	139.2 psf	197.9 psf	295.8 psf	192.8 psf	0.0 psf	0.0 psf	0.0 psf
$f_{\text{max}}$	422.6 psf	441.5 psf	421.9 psf	411.1 psf	507.3 psf	413.9 psf	252.8 psf	247.5 psf	254.8 psf



Maximum Bearing Pressure = 507 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

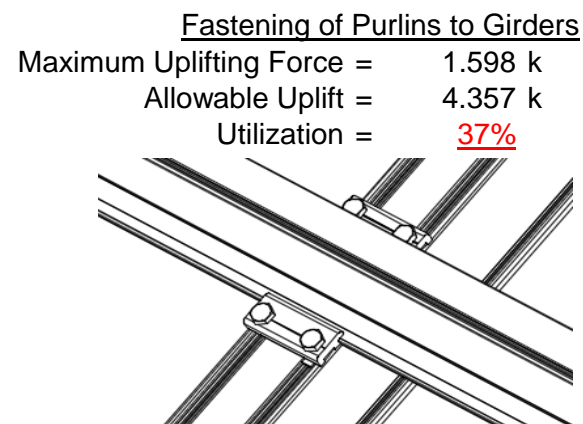
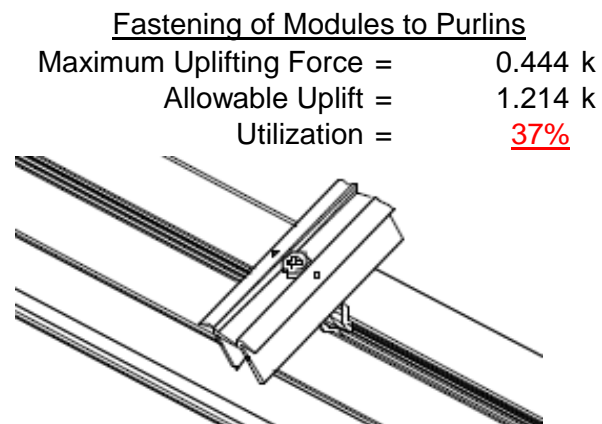
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

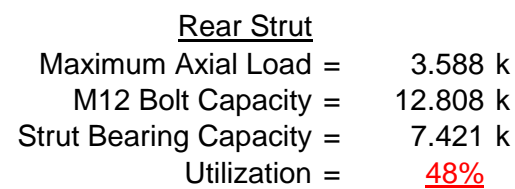
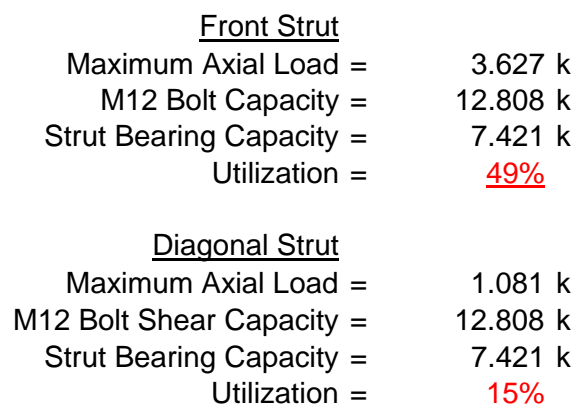
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



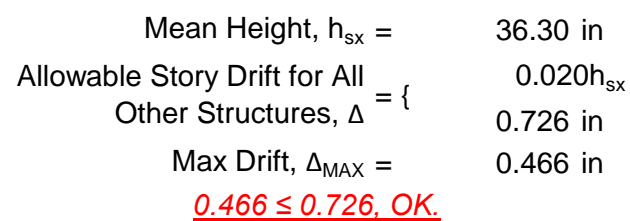
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 123 \text{ in}$$

$$J = 0.432$$

$$340.276$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.3 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 123$$

$$J = 0.432$$

$$216.395$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-57.906	-57.906	0	0
2	M14	y	-57.906	-57.906	0	0
3	M15	y	-92.65	-92.65	0	0
4	M16	y	-92.65	-92.65	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	133.185	133.185	0	0
2	M14	y	103.073	103.073	0	0
3	M15	y	57.906	57.906	0	0
4	M16	y	57.906	57.906	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



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Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	88.769	1	868.951	1	-4.622	12	.005	14	.268	1	1.825	1
20		min	3.498	12	-820.19	3	-150.892	1	-.013	1	.007	12	-1.682	3
21	11	max	88.769	1	715.414	1	-3.569	12	.013	1	.114	1	.923	1
22		min	3.498	12	-674.402	3	-118.587	1	0	3	.002	12	-.831	3
23	12	max	88.769	1	561.878	1	-2.516	12	.013	1	.035	4	.195	1
24		min	3.498	12	-528.614	3	-86.283	1	0	3	-.002	1	-.146	3
25	13	max	88.769	1	408.342	1	-1.463	12	.013	1	.016	5	.373	3
26		min	3.498	12	-382.826	3	-53.978	1	0	3	-.082	1	-.357	1
27	14	max	88.769	1	254.805	1	-.41	12	.013	1	0	15	.726	3
28		min	.768	15	-237.038	3	-21.674	1	0	3	-.125	1	-.735	1
29	15	max	88.769	1	101.269	1	10.631	1	.013	1	-.004	12	.913	3
30		min	-10.431	5	-91.25	3	-12.477	5	0	3	-.132	1	-.938	1
31	16	max	88.769	1	54.538	3	42.935	1	.013	1	-.003	12	.933	3
32		min	-22.122	5	-52.268	1	-10.848	5	0	3	-.101	1	-.965	1
33	17	max	88.769	1	200.325	3	75.24	1	.013	1	0	12	.788	3
34		min	-33.813	5	-205.804	1	-9.219	5	0	3	-.049	4	-.818	1
35	18	max	88.769	1	346.113	3	107.544	1	.013	1	.07	1	.477	3
36		min	-45.504	5	-359.34	1	-7.59	5	0	3	-.051	5	-.497	1
37	19	max	88.769	1	491.901	3	139.849	1	.013	1	.211	1	0	1
38		min	-57.195	5	-512.877	1	-5.96	5	0	3	-.058	5	0	3
39	M14	1	max	63.36	4	540.735	1	-4.99	12	.006	.24	1	0	1
40		min	1.507	12	-386.579	3	-144.087	1	-.011	1	.009	12	0	3
41	2	max	51.669	4	387.199	1	-3.937	12	.006	3	.104	4	.377	3
42		min	1.507	12	-275.085	3	-111.783	1	-.011	1	.004	12	-.528	1
43	3	max	40.774	1	233.662	1	-2.883	12	.006	3	.056	5	.627	3
44		min	1.507	12	-163.592	3	-79.478	1	-.011	1	-.015	1	-.882	1
45	4	max	40.774	1	80.126	1	-1.83	12	.006	3	.03	5	.749	3
46		min	1.507	12	-52.098	3	-47.173	1	-.011	1	-.087	1	-1.061	1
47	5	max	40.774	1	59.396	3	-.777	12	.006	3	.006	5	.745	3
48		min	1.507	12	-73.411	1	-23.778	4	-.011	1	-.122	1	-1.064	1
49	6	max	40.774	1	170.889	3	17.436	1	.006	3	-.004	12	.614	3
50		min	-3.427	5	-226.947	1	-18.655	5	-.011	1	-.121	1	-.893	1
51	7	max	40.774	1	282.383	3	49.74	1	.006	3	-.003	12	.356	3
52		min	-15.118	5	-380.483	1	-17.026	5	-.011	1	-.082	1	-.548	1
53	8	max	40.774	1	393.876	3	82.045	1	.006	3	0	10	0	15
54		min	-26.808	5	-534.02	1	-15.397	5	-.011	1	-.058	4	-.033	2
55	9	max	40.774	1	505.37	3	114.349	1	.006	3	.104	1	.669	1
56		min	-38.499	5	-687.556	1	-13.767	5	-.011	1	-.072	5	-.541	3
57	10	max	62.173	4	841.093	1	-4.488	12	.006	3	.253	1	1.539	1
58		min	1.507	12	-616.864	3	-146.654	1	-.011	1	.007	12	-1.18	3
59	11	max	50.482	4	687.556	1	-3.435	12	.011	1	.105	4	.669	1
60		min	1.507	12	-505.37	3	-114.349	1	-.006	3	.002	12	-.541	3
61	12	max	40.774	1	534.02	1	-2.382	12	.011	1	.054	5	0	15
62		min	1.507	12	-393.876	3	-82.045	1	-.006	3	-.007	1	-.033	2
63	13	max	40.774	1	380.483	1	-1.329	12	.011	1	.028	5	.356	3
64		min	1.507	12	-282.383	3	-49.74	1	-.006	3	-.082	1	-.548	1
65	14	max	40.774	1	226.947	1	-.276	12	.011	1	.004	5	.614	3
66		min	1.507	12	-170.889	3	-24.335	4	-.006	3	-.121	1	-.893	1
67	15	max	40.774	1	73.411	1	14.869	1	.011	1	-.004	12	.745	3
68		min	-4.325	5	-59.396	3	-18.768	5	-.006	3	-.122	1	-1.064	1
69	16	max	40.774	1	52.098	3	47.173	1	.011	1	-.002	12	.749	3
70		min	-16.016	5	-80.126	1	-17.139	5	-.006	3	-.087	1	-1.061	1
71	17	max	40.774	1	163.592	3	79.478	1	.011	1	0	3	.627	3
72		min	-27.707	5	-233.662	1	-15.509	5	-.006	3	-.061	4	-.882	1
73	18	max	40.774	1	275.085	3	111.783	1	.011	1	.094	1	.377	3
74		min	-39.397	5	-387.199	1	-13.88	5	-.006	3	-.074	5	-.528	1
75	19	max	40.774	1	386.579	3	144.087	1	.011	1	.24	1	0	1



Company : Schletter, Inc.  
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Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC	
76	M15	1	min	-51.088	5	-540.735	1	-12.251	5	-.006	3	-.089	5	0	3	
77			max	80.277	5	607.5	1	-4.967	12	.012	1	.24	1	0	2	
78			min	-42.852	1	-213.138	3	-144.067	1	-.005	3	.009	12	0	3	
79			2	max	68.586	5	434.179	1	-3.914	12	.012	1	.14	4	.209	3
80				min	-42.852	1	-153.084	3	-111.762	1	-.005	3	.004	12	-.593	1
81			3	max	56.895	5	260.858	1	-2.861	12	.012	1	.081	5	.349	3
82				min	-42.852	1	-93.031	3	-79.458	1	-.005	3	-.015	1	-.989	1
83			4	max	45.204	5	87.537	1	-1.808	12	.012	1	.045	5	.42	3
84				min	-42.852	1	-32.977	3	-47.153	1	-.005	3	-.087	1	-1.187	1
85			5	max	33.513	5	27.076	3	-.755	12	.012	1	.011	5	.424	3
86				min	-42.852	1	-85.784	1	-32.517	4	-.005	3	-.122	1	-1.188	1
87			6	max	21.822	5	87.129	3	17.456	1	.012	1	-.004	12	.359	3
88	min			-42.852	1	-259.106	1	-27.392	5	-.005	3	-.121	1	-.992	1	
89		7	max	10.131	5	147.183	3	49.76	1	.012	1	-.003	12	.225	3	
90			min	-42.852	1	-432.427	1	-25.763	5	-.005	3	-.082	1	-.598	1	
91		8	max	-1.007	15	207.236	3	82.065	1	.012	1	0	10	.024	3	
92			min	-42.852	1	-605.748	1	-24.133	5	-.005	3	-.082	4	-.009	9	
93		9	max	-1.724	12	267.29	3	114.369	1	.012	1	.104	1	.782	1	
94			min	-42.852	1	-779.069	1	-22.504	5	-.005	3	-.106	5	-.247	3	
95		10	max	-1.724	12	952.39	1	-4.511	12	.012	1	.253	1	1.768	1	
96			min	-42.852	1	-327.343	3	-146.674	1	-.005	3	.007	12	-.585	3	
97		11	max	8.747	5	779.069	1	-3.458	12	.005	3	.139	4	.782	1	
98			min	-42.852	1	-267.29	3	-114.369	1	-.012	1	.002	12	-.247	3	
99		12	max	-1.724	12	605.748	1	-2.404	12	.005	3	.079	5	.024	3	
100			min	-42.852	1	-207.236	3	-82.065	1	-.012	1	-.007	1	-.009	9	
101		13	max	-1.724	12	432.427	1	-1.351	12	.005	3	.043	5	.225	3	
102			min	-42.852	1	-147.183	3	-49.76	1	-.012	1	-.082	1	-.598	1	
103		14	max	-1.724	12	259.106	1	-.298	12	.005	3	.009	5	.359	3	
104			min	-42.852	1	-87.129	3	-33.087	4	-.012	1	-.121	1	-.992	1	
105		15	max	-1.724	12	85.784	1	14.849	1	.005	3	-.004	12	.424	3	
106			min	-47.149	4	-27.076	3	-27.506	5	-.012	1	-.122	1	-1.188	1	
107		16	max	-1.724	12	32.977	3	47.153	1	.005	3	-.002	12	.42	3	
108			min	-58.84	4	-87.537	1	-25.876	5	-.012	1	-.087	1	-1.187	1	
109		17	max	-1.724	12	93.031	3	79.458	1	.005	3	0	3	.349	3	
110			min	-70.531	4	-260.858	1	-24.247	5	-.012	1	-.087	4	-.989	1	
111		18	max	-1.724	12	153.084	3	111.762	1	.005	3	.094	1	.209	3	
112			min	-82.222	4	-434.179	1	-22.618	5	-.012	1	-.109	5	-.593	1	
113		19	max	-1.724	12	213.138	3	144.067	1	.005	3	.24	1	0	2	
114			min	-93.913	4	-607.5	1	-20.989	5	-.012	1	-.134	5	0	5	
115	M16	1	max	80.103	5	579.902	1	-4.782	12	.012	1	.212	1	0	1	
116			min	-93.666	1	-199.775	3	-140.021	1	-.007	3	.008	12	0	3	
117		2	max	68.412	5	406.581	1	-3.729	12	.012	1	.104	4	.193	3	
118			min	-93.666	1	-139.722	3	-107.716	1	-.007	3	.003	12	-.562	1	
119		3	max	56.721	5	233.259	1	-2.676	12	.012	1	.06	5	.318	3	
120			min	-93.666	1	-79.668	3	-75.412	1	-.007	3	-.033	1	-.926	1	
121		4	max	45.03	5	59.938	1	-1.623	12	.012	1	.033	5	.375	3	
122			min	-93.666	1	-19.615	3	-43.107	1	-.007	3	-.101	1	-1.093	1	
123		5	max	33.339	5	40.439	3	-.57	12	.012	1	.008	5	.363	3	
124			min	-93.666	1	-113.383	1	-23.357	4	-.007	3	-.131	1	-1.063	1	
125		6	max	21.648	5	100.492	3	21.502	1	.012	1	-.004	12	.283	3	
126			min	-93.666	1	-286.704	1	-19.185	5	-.007	3	-.125	1	-.835	1	
127		7	max	9.958	5	160.546	3	53.807	1	.012	1	-.003	12	.134	3	
128			min	-93.666	1	-460.025	1	-17.556	5	-.007	3	-.082	1	-.41	1	
129		8	max	-1.1	15	220.599	3	86.111	1	.012	1	0	10	.213	1	
130			min	-93.666	1	-633.346	1	-15.927	5	-.007	3	-.056	4	-.083	3	
131		9	max	-3.549	12	280.652	3	118.416	1	.012	1	.114	1	1.033	1	
132			min	-93.666	1	-806.668	1	-14.297	5	-.007	3	-.072	5	-.368	3	





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-3.549	12	979.989	1	-4.696	12	.012	1	.267	1	2.05	1
134			min	-93.666	1	-340.706	3	-150.72	1	-.007	3	.008	12	-.722	3
135		11	max	4.006	5	806.668	1	-3.643	12	.007	3	.114	1	1.033	1
136			min	-93.666	1	-280.652	3	-118.416	1	-.012	1	.003	12	-.368	3
137		12	max	-3.549	12	633.346	1	-2.59	12	.007	3	.054	4	.213	1
138			min	-93.666	1	-220.599	3	-86.111	1	-.012	1	-.003	1	-.083	3
139		13	max	-3.549	12	460.025	1	-1.536	12	.007	3	.027	5	.134	3
140			min	-93.666	1	-160.546	3	-53.807	1	-.012	1	-.082	1	-.41	1
141		14	max	-3.549	12	286.704	1	-.483	12	.007	3	.002	5	.283	3
142			min	-93.666	1	-100.492	3	-25.97	4	-.012	1	-.125	1	-.835	1
143		15	max	-3.549	12	113.383	1	10.803	1	.007	3	-.004	12	.363	3
144			min	-93.666	1	-40.439	3	-19.715	5	-.012	1	-.131	1	-1.063	1
145		16	max	-3.549	12	19.615	3	43.107	1	.007	3	-.003	12	.375	3
146			min	-93.666	1	-59.938	1	-18.086	5	-.012	1	-.101	1	-1.093	1
147		17	max	-3.549	12	79.668	3	75.412	1	.007	3	0	12	.318	3
148			min	-93.666	1	-233.259	1	-16.457	5	-.012	1	-.071	4	-.926	1
149		18	max	-3.549	12	139.722	3	107.716	1	.007	3	.071	1	.193	3
150			min	-97.478	4	-406.581	1	-14.827	5	-.012	1	-.081	5	-.562	1
151		19	max	-3.549	12	199.775	3	140.021	1	.007	3	.212	1	0	1
152			min	-109.169	4	-579.902	1	-13.198	5	-.012	1	-.097	5	0	5
153	M2	1	max	1110.913	1	2.332	4	1.261	1	0	3	0	3	0	1
154			min	-866.293	3	.571	15	-80.663	4	0	4	0	1	0	1
155		2	max	1111.242	1	2.316	4	1.261	1	0	3	0	1	0	15
156			min	-866.047	3	.568	15	-80.948	4	0	4	-.018	4	0	4
157		3	max	1111.57	1	2.301	4	1.261	1	0	3	0	1	0	15
158			min	-865.8	3	.564	15	-81.233	4	0	4	-.036	4	-.001	4
159		4	max	1111.898	1	2.286	4	1.261	1	0	3	0	1	0	15
160			min	-865.554	3	.561	15	-81.518	4	0	4	-.054	4	-.002	4
161		5	max	1112.227	1	2.271	4	1.261	1	0	3	.001	1	0	15
162			min	-865.308	3	.557	15	-81.803	4	0	4	-.072	4	-.002	4
163		6	max	1112.555	1	2.255	4	1.261	1	0	3	.001	1	0	15
164			min	-865.061	3	.553	15	-82.087	4	0	4	-.09	4	-.003	4
165		7	max	1112.884	1	2.24	4	1.261	1	0	3	.002	1	0	15
166			min	-864.815	3	.55	15	-82.372	4	0	4	-.108	4	-.003	4
167		8	max	1113.212	1	2.225	4	1.261	1	0	3	.002	1	0	15
168			min	-864.569	3	.546	15	-82.657	4	0	4	-.127	4	-.004	4
169		9	max	1113.541	1	2.21	4	1.261	1	0	3	.002	1	0	15
170			min	-864.322	3	.543	15	-82.942	4	0	4	-.145	4	-.004	4
171		10	max	1113.869	1	2.194	4	1.261	1	0	3	.002	1	-.001	15
172			min	-864.076	3	.539	15	-83.227	4	0	4	-.163	4	-.005	4
173		11	max	1114.198	1	2.179	4	1.261	1	0	3	.003	1	-.001	15
174			min	-863.83	3	.536	15	-83.511	4	0	4	-.182	4	-.005	4
175		12	max	1114.526	1	2.164	4	1.261	1	0	3	.003	1	-.001	15
176			min	-863.583	3	.532	15	-83.796	4	0	4	-.2	4	-.005	4
177		13	max	1114.854	1	2.149	4	1.261	1	0	3	.003	1	-.001	15
178			min	-863.337	3	.528	15	-84.081	4	0	4	-.219	4	-.006	4
179		14	max	1115.183	1	2.133	4	1.261	1	0	3	.004	1	-.002	15
180			min	-863.091	3	.525	15	-84.366	4	0	4	-.238	4	-.006	4
181		15	max	1115.511	1	2.118	4	1.261	1	0	3	.004	1	-.002	15
182			min	-862.844	3	.521	15	-84.651	4	0	4	-.256	4	-.007	4
183		16	max	1115.84	1	2.103	4	1.261	1	0	3	.004	1	-.002	15
184			min	-862.598	3	.518	15	-84.936	4	0	4	-.275	4	-.007	4
185		17	max	1116.168	1	2.087	4	1.261	1	0	3	.004	1	-.002	15
186			min	-862.352	3	.514	15	-85.22	4	0	4	-.294	4	-.008	4
187		18	max	1116.497	1	2.072	4	1.261	1	0	3	.005	1	-.002	15
188			min	-862.105	3	.51	15	-85.505	4	0	4	-.313	4	-.008	4
189		19	max	1116.825	1	2.057	4	1.261	1	0	3	.005	1	-.002	15



Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-861.859	3	.507	15	-85.79	4	0	4	-.332	4	-.009	4
191	M3	1	max	236.964	2	8.105	4	.012	1	0	3	0	.009	4
192		min	-344.146	3	1.917	15	-1.215	5	0	4	-.012	4	.002	15
193		2	max	236.793	2	7.332	4	.012	1	0	3	0	.005	4
194		min	-344.274	3	1.736	15	-.673	5	0	4	-.013	4	.001	15
195		3	max	236.623	2	6.56	4	.012	1	0	3	0	.003	2
196		min	-344.401	3	1.554	15	-.131	5	0	4	-.013	4	0	12
197		4	max	236.453	2	5.787	4	.465	4	0	3	0	0	2
198		min	-344.529	3	1.373	15	0	12	0	4	-.013	4	-.001	3
199		5	max	236.282	2	5.015	4	1.007	4	0	3	0	0	15
200		min	-344.657	3	1.191	15	0	12	0	4	-.013	4	-.002	3
201		6	max	236.112	2	4.243	4	1.549	4	0	3	0	0	15
202		min	-344.785	3	1.01	15	0	12	0	4	-.012	4	-.004	6
203		7	max	235.941	2	3.47	4	2.091	4	0	3	0	1	15
204		min	-344.912	3	.828	15	0	12	0	4	-.011	4	-.006	6
205		8	max	235.771	2	2.698	4	2.633	4	0	3	0	1	15
206		min	-345.04	3	.646	15	0	12	0	4	-.01	4	-.007	6
207		9	max	235.601	2	1.925	4	3.175	4	0	3	0	1	15
208		min	-345.168	3	.465	15	0	12	0	4	-.009	4	-.008	6
209		10	max	235.43	2	1.153	4	3.718	4	0	3	0	1	15
210		min	-345.296	3	.283	15	0	12	0	4	-.008	5	-.009	6
211		11	max	235.26	2	.383	2	4.26	4	0	3	0	1	15
212		min	-345.423	3	.048	12	0	12	0	4	-.006	5	-.009	6
213		12	max	235.09	2	-.08	15	4.802	4	0	3	0	1	15
214		min	-345.551	3	-.395	3	0	12	0	4	-.004	5	-.009	6
215		13	max	234.919	2	-.261	15	5.344	4	0	3	0	1	15
216		min	-345.679	3	-1.165	6	0	12	0	4	-.002	5	-.009	6
217		14	max	234.749	2	-.443	15	5.886	4	0	3	0	4	15
218		min	-345.807	3	-1.938	6	0	12	0	4	0	12	-.008	6
219		15	max	234.579	2	-.625	15	6.428	4	0	3	.003	4	15
220		min	-345.934	3	-2.71	6	0	12	0	4	0	12	-.007	6
221		16	max	234.408	2	-.806	15	6.97	4	0	3	.006	4	15
222		min	-346.062	3	-3.483	6	0	12	0	4	0	12	-.006	6
223		17	max	234.238	2	-.988	15	7.512	4	0	3	.009	4	15
224		min	-346.19	3	-4.255	6	0	12	0	4	0	12	-.004	6
225		18	max	234.068	2	-1.169	15	8.055	4	0	3	.012	4	15
226		min	-346.318	3	-5.028	6	0	12	0	4	0	12	-.002	6
227		19	max	233.897	2	-1.351	15	8.597	4	0	3	.016	4	1
228		min	-346.445	3	-5.8	6	0	12	0	4	0	12	0	1
229	M4	1	max	1235.065	1	0	1	-.317	12	0	1	.008	4	1
230		min	-256.873	3	0	1	-240.299	4	0	1	0	10	0	1
231		2	max	1235.236	1	0	1	-.317	12	0	1	0	12	1
232		min	-256.745	3	0	1	-240.447	4	0	1	-.019	4	0	1
233		3	max	1235.406	1	0	1	-.317	12	0	1	0	12	1
234		min	-256.617	3	0	1	-240.594	4	0	1	-.047	4	0	1
235		4	max	1235.576	1	0	1	-.317	12	0	1	0	12	1
236		min	-256.49	3	0	1	-240.742	4	0	1	-.075	4	0	1
237		5	max	1235.747	1	0	1	-.317	12	0	1	0	12	1
238		min	-256.362	3	0	1	-240.89	4	0	1	-.102	4	0	1
239		6	max	1235.917	1	0	1	-.317	12	0	1	0	12	1
240		min	-256.234	3	0	1	-241.037	4	0	1	-.13	4	0	1
241		7	max	1236.087	1	0	1	-.317	12	0	1	0	12	1
242		min	-256.106	3	0	1	-241.185	4	0	1	-.158	4	0	1
243		8	max	1236.258	1	0	1	-.317	12	0	1	0	12	1
244		min	-255.979	3	0	1	-241.333	4	0	1	-.185	4	0	1
245		9	max	1236.428	1	0	1	-.317	12	0	1	0	12	1
246		min	-255.851	3	0	1	-241.48	4	0	1	-.213	4	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1236.599	1	0	1	-.317	12	0	1	0	12	0	1
248			min	-255.723	3	0	1	-241.628	4	0	1	-.241	4	0	1
249		11	max	1236.769	1	0	1	-.317	12	0	1	0	12	0	1
250			min	-255.595	3	0	1	-241.775	4	0	1	-.268	4	0	1
251		12	max	1236.939	1	0	1	-.317	12	0	1	0	12	0	1
252			min	-255.467	3	0	1	-241.923	4	0	1	-.296	4	0	1
253		13	max	1237.11	1	0	1	-.317	12	0	1	0	12	0	1
254			min	-255.34	3	0	1	-242.071	4	0	1	-.324	4	0	1
255		14	max	1237.28	1	0	1	-.317	12	0	1	0	12	0	1
256			min	-255.212	3	0	1	-242.218	4	0	1	-.352	4	0	1
257		15	max	1237.45	1	0	1	-.317	12	0	1	0	12	0	1
258			min	-255.084	3	0	1	-242.366	4	0	1	-.38	4	0	1
259		16	max	1237.621	1	0	1	-.317	12	0	1	0	12	0	1
260			min	-254.956	3	0	1	-242.514	4	0	1	-.407	4	0	1
261		17	max	1237.791	1	0	1	-.317	12	0	1	0	12	0	1
262			min	-254.829	3	0	1	-242.661	4	0	1	-.435	4	0	1
263		18	max	1237.961	1	0	1	-.317	12	0	1	0	12	0	1
264			min	-254.701	3	0	1	-242.809	4	0	1	-.463	4	0	1
265		19	max	1238.132	1	0	1	-.317	12	0	1	0	12	0	1
266			min	-254.573	3	0	1	-242.956	4	0	1	-.491	4	0	1
267	M6	1	max	3582.273	1	2.583	2	0	1	0	1	0	4	0	1
268			min	-2846.935	3	.338	12	-81.388	4	0	4	0	1	0	1
269		2	max	3582.601	1	2.571	2	0	1	0	1	0	1	0	12
270			min	-2846.688	3	.332	12	-81.673	4	0	4	-.018	4	0	2
271		3	max	3582.93	1	2.559	2	0	1	0	1	0	1	0	12
272			min	-2846.442	3	.326	12	-81.957	4	0	4	-.036	4	-.001	2
273		4	max	3583.258	1	2.547	2	0	1	0	1	0	1	0	12
274			min	-2846.196	3	.32	12	-82.242	4	0	4	-.054	4	-.002	2
275		5	max	3583.586	1	2.536	2	0	1	0	1	0	1	0	12
276			min	-2845.949	3	.314	12	-82.527	4	0	4	-.073	4	-.002	2
277		6	max	3583.915	1	2.524	2	0	1	0	1	0	1	0	12
278			min	-2845.703	3	.308	12	-82.812	4	0	4	-.091	4	-.003	2
279		7	max	3584.243	1	2.512	2	0	1	0	1	0	1	0	12
280			min	-2845.457	3	.302	12	-83.097	4	0	4	-.109	4	-.003	2
281		8	max	3584.572	1	2.5	2	0	1	0	1	0	1	0	12
282			min	-2845.21	3	.296	12	-83.382	4	0	4	-.128	4	-.004	2
283		9	max	3584.9	1	2.488	2	0	1	0	1	0	1	0	12
284			min	-2844.964	3	.29	12	-83.666	4	0	4	-.146	4	-.004	2
285		10	max	3585.229	1	2.476	2	0	1	0	1	0	1	0	12
286			min	-2844.718	3	.284	12	-83.951	4	0	4	-.165	4	-.005	2
287		11	max	3585.557	1	2.464	2	0	1	0	1	0	1	0	12
288			min	-2844.471	3	.278	12	-84.236	4	0	4	-.183	4	-.006	2
289		12	max	3585.885	1	2.452	2	0	1	0	1	0	1	0	12
290			min	-2844.225	3	.273	12	-84.521	4	0	4	-.202	4	-.006	2
291		13	max	3586.214	1	2.44	2	0	1	0	1	0	1	0	12
292			min	-2843.979	3	.267	12	-84.806	4	0	4	-.221	4	-.007	2
293		14	max	3586.542	1	2.429	2	0	1	0	1	0	1	0	12
294			min	-2843.732	3	.261	12	-85.091	4	0	4	-.24	4	-.007	2
295		15	max	3586.871	1	2.417	2	0	1	0	1	0	1	0	12
296			min	-2843.486	3	.255	12	-85.375	4	0	4	-.259	4	-.008	2
297		16	max	3587.199	1	2.405	2	0	1	0	1	0	1	0	12
298			min	-2843.24	3	.249	12	-85.66	4	0	4	-.278	4	-.008	2
299		17	max	3587.528	1	2.393	2	0	1	0	1	0	1	-.001	12
300			min	-2842.993	3	.243	12	-85.945	4	0	4	-.297	4	-.009	2
301		18	max	3587.856	1	2.381	2	0	1	0	1	0	1	-.001	12
302			min	-2842.747	3	.237	12	-86.23	4	0	4	-.316	4	-.009	2
303		19	max	3588.185	1	2.369	2	0	1	0	1	0	1	-.001	12



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Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-2842.501	3	.231	12	-86.515	4	0	4	-.335	4	-.01	2
305	M7	1	max	1022.291	2	8.119	6	0	1	0	1	0	.01	2
306		min	-1078.222	3	1.905	15	-1.285	5	0	4	-.013	4	.001	12
307		2	max	1022.12	2	7.346	6	0	1	0	1	0	.007	2
308		min	-1078.35	3	1.723	15	-.743	5	0	4	-.013	4	0	3
309		3	max	1021.95	2	6.574	6	0	1	0	1	0	.005	2
310		min	-1078.477	3	1.542	15	-.201	5	0	4	-.013	4	-.002	3
311		4	max	1021.78	2	5.801	6	.387	4	0	1	0	.003	2
312		min	-1078.605	3	1.36	15	0	1	0	4	-.013	4	-.003	3
313		5	max	1021.609	2	5.029	6	.929	4	0	1	0	0	2
314		min	-1078.733	3	1.178	15	0	1	0	4	-.013	4	-.004	3
315		6	max	1021.439	2	4.257	6	1.471	4	0	1	0	1	15
316		min	-1078.861	3	.997	15	0	1	0	4	-.012	4	-.005	3
317		7	max	1021.268	2	3.484	6	2.013	4	0	1	0	1	15
318		min	-1078.988	3	.815	15	0	1	0	4	-.012	4	-.006	3
319		8	max	1021.098	2	2.712	6	2.556	4	0	1	0	1	15
320		min	-1079.116	3	.634	15	0	1	0	4	-.011	4	-.007	4
321		9	max	1020.928	2	1.939	6	3.098	4	0	1	0	1	15
322		min	-1079.244	3	.45	12	0	1	0	4	-.009	4	-.008	4
323		10	max	1020.757	2	1.301	2	3.64	4	0	1	0	1	15
324		min	-1079.372	3	.149	12	0	1	0	4	-.008	4	-.009	4
325		11	max	1020.587	2	.699	2	4.182	4	0	1	0	1	15
326		min	-1079.499	3	-.276	3	0	1	0	4	-.006	4	-.009	4
327		12	max	1020.417	2	.097	2	4.724	4	0	1	0	1	15
328		min	-1079.627	3	-.728	3	0	1	0	4	-.005	4	-.009	4
329		13	max	1020.246	2	-.274	15	5.266	4	0	1	0	1	15
330		min	-1079.755	3	-1.179	3	0	1	0	4	-.002	4	-.009	4
331		14	max	1020.076	2	-.456	15	5.808	4	0	1	0	1	15
332		min	-1079.883	3	-1.923	4	0	1	0	4	0	5	-.008	4
333		15	max	1019.906	2	-.637	15	6.35	4	0	1	.002	4	15
334		min	-1080.01	3	-2.695	4	0	1	0	4	0	1	-.007	4
335		16	max	1019.735	2	-.819	15	6.893	4	0	1	.005	4	15
336		min	-1080.138	3	-3.468	4	0	1	0	4	0	1	-.006	4
337		17	max	1019.565	2	-.1	15	7.435	4	0	1	.008	4	15
338		min	-1080.266	3	-4.24	4	0	1	0	4	0	1	-.004	4
339		18	max	1019.395	2	-1.182	15	7.977	4	0	1	.012	4	15
340		min	-1080.394	3	-5.012	4	0	1	0	4	0	1	-.002	4
341		19	max	1019.224	2	-1.363	15	8.519	4	0	1	.015	4	1
342		min	-1080.522	3	-5.785	4	0	1	0	4	0	1	0	1
343	M8	1	max	3623.69	1	0	1	0	1	0	1	.008	4	1
344		min	-890.242	3	0	1	-234.915	4	0	1	0	1	0	1
345		2	max	3623.861	1	0	1	0	1	0	1	0	1	1
346		min	-890.114	3	0	1	-235.062	4	0	1	-.019	4	0	1
347		3	max	3624.031	1	0	1	0	1	0	1	0	1	1
348		min	-889.987	3	0	1	-235.21	4	0	1	-.046	4	0	1
349		4	max	3624.201	1	0	1	0	1	0	1	0	1	1
350		min	-889.859	3	0	1	-235.358	4	0	1	-.073	4	0	1
351		5	max	3624.372	1	0	1	0	1	0	1	0	1	1
352		min	-889.731	3	0	1	-235.505	4	0	1	-.1	4	0	1
353		6	max	3624.542	1	0	1	0	1	0	1	0	1	1
354		min	-889.603	3	0	1	-235.653	4	0	1	-.127	4	0	1
355		7	max	3624.712	1	0	1	0	1	0	1	0	1	1
356		min	-889.476	3	0	1	-235.801	4	0	1	-.154	4	0	1
357		8	max	3624.883	1	0	1	0	1	0	1	0	1	1
358		min	-889.348	3	0	1	-235.948	4	0	1	-.181	4	0	1
359		9	max	3625.053	1	0	1	0	1	0	1	0	1	1
360		min	-889.22	3	0	1	-236.096	4	0	1	-.208	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361	10	max	3625.224	1	0	1	0	1	0	1	0	1	0	1
362		min	-889.092	3	0	1	-236.243	4	0	1	-.235	4	0	1
363	11	max	3625.394	1	0	1	0	1	0	1	0	1	0	1
364		min	-888.964	3	0	1	-236.391	4	0	1	-.263	4	0	1
365	12	max	3625.564	1	0	1	0	1	0	1	0	1	0	1
366		min	-888.837	3	0	1	-236.539	4	0	1	-.29	4	0	1
367	13	max	3625.735	1	0	1	0	1	0	1	0	1	0	1
368		min	-888.709	3	0	1	-236.686	4	0	1	-.317	4	0	1
369	14	max	3625.905	1	0	1	0	1	0	1	0	1	0	1
370		min	-888.581	3	0	1	-236.834	4	0	1	-.344	4	0	1
371	15	max	3626.075	1	0	1	0	1	0	1	0	1	0	1
372		min	-888.453	3	0	1	-236.982	4	0	1	-.371	4	0	1
373	16	max	3626.246	1	0	1	0	1	0	1	0	1	0	1
374		min	-888.326	3	0	1	-237.129	4	0	1	-.399	4	0	1
375	17	max	3626.416	1	0	1	0	1	0	1	0	1	0	1
376		min	-888.198	3	0	1	-237.277	4	0	1	-.426	4	0	1
377	18	max	3626.586	1	0	1	0	1	0	1	0	1	0	1
378		min	-888.07	3	0	1	-237.425	4	0	1	-.453	4	0	1
379	19	max	3626.757	1	0	1	0	1	0	1	0	1	0	1
380		min	-887.942	3	0	1	-237.572	4	0	1	-.48	4	0	1
381	M10	1	max	1110.913	1	2.229	6	-.046	12	0	1	0	1	0
382		min	-866.293	3	.503	15	-81.296	4	0	5	0	3	0	1
383	2	max	1111.242	1	2.214	6	-.046	12	0	1	0	10	0	15
384		min	-866.047	3	.499	15	-81.581	4	0	5	-.018	4	0	6
385	3	max	1111.57	1	2.199	6	-.046	12	0	1	0	12	0	15
386		min	-865.8	3	.496	15	-81.865	4	0	5	-.036	4	0	6
387	4	max	1111.898	1	2.184	6	-.046	12	0	1	0	12	0	15
388		min	-865.554	3	.492	15	-82.15	4	0	5	-.054	4	-.001	6
389	5	max	1112.227	1	2.168	6	-.046	12	0	1	0	12	0	15
390		min	-865.308	3	.489	15	-82.435	4	0	5	-.073	4	-.002	6
391	6	max	1112.555	1	2.153	6	-.046	12	0	1	0	12	0	15
392		min	-865.061	3	.485	15	-82.72	4	0	5	-.091	4	-.002	6
393	7	max	1112.884	1	2.138	6	-.046	12	0	1	0	12	0	15
394		min	-864.815	3	.481	15	-83.005	4	0	5	-.109	4	-.003	6
395	8	max	1113.212	1	2.123	6	-.046	12	0	1	0	12	0	15
396		min	-864.569	3	.478	15	-83.29	4	0	5	-.128	4	-.003	6
397	9	max	1113.541	1	2.107	6	-.046	12	0	1	0	12	0	15
398		min	-864.322	3	.474	15	-83.574	4	0	5	-.146	4	-.004	6
399	10	max	1113.869	1	2.092	6	-.046	12	0	1	0	12	0	15
400		min	-864.076	3	.471	15	-83.859	4	0	5	-.165	4	-.004	6
401	11	max	1114.198	1	2.077	6	-.046	12	0	1	0	12	-.001	15
402		min	-863.83	3	.467	15	-84.144	4	0	5	-.183	4	-.005	6
403	12	max	1114.526	1	2.062	6	-.046	12	0	1	0	12	-.001	15
404		min	-863.583	3	.463	15	-84.429	4	0	5	-.202	4	-.005	6
405	13	max	1114.854	1	2.046	6	-.046	12	0	1	0	12	-.001	15
406		min	-863.337	3	.46	15	-84.714	4	0	5	-.221	4	-.006	6
407	14	max	1115.183	1	2.031	6	-.046	12	0	1	0	12	-.001	15
408		min	-863.091	3	.456	15	-84.999	4	0	5	-.239	4	-.006	6
409	15	max	1115.511	1	2.016	6	-.046	12	0	1	0	12	-.001	15
410		min	-862.844	3	.453	15	-85.283	4	0	5	-.258	4	-.007	6
411	16	max	1115.84	1	2.001	6	-.046	12	0	1	0	12	-.002	15
412		min	-862.598	3	.449	15	-85.568	4	0	5	-.277	4	-.007	6
413	17	max	1116.168	1	1.985	6	-.046	12	0	1	0	12	-.002	15
414		min	-862.352	3	.445	15	-85.853	4	0	5	-.296	4	-.007	6
415	18	max	1116.497	1	1.97	6	-.046	12	0	1	0	12	-.002	15
416		min	-862.105	3	.442	15	-86.138	4	0	5	-.315	4	-.008	6
417	19	max	1116.825	1	1.955	6	-.046	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418		min	-861.859	3	.438	15	-86.423	4	0	5	-.334	4	-.008	6
419	M11	1	max	236.964	2	8.051	6	0	12	0	1	0	.008	6
420		min	-344.146	3	1.881	15	-1.216	5	0	4	-.013	4	.002	15
421		2	max	236.793	2	7.279	6	0	12	0	1	0	.005	6
422		min	-344.274	3	1.7	15	-.674	5	0	4	-.013	4	.001	15
423		3	max	236.623	2	6.506	6	0	12	0	1	0	.003	2
424		min	-344.401	3	1.518	15	-.132	5	0	4	-.013	4	0	12
425		4	max	236.453	2	5.734	6	.459	4	0	1	0	12	2
426		min	-344.529	3	1.337	15	-.012	1	0	4	-.013	4	-.001	3
427		5	max	236.282	2	4.961	6	1.001	4	0	1	0	12	15
428		min	-344.657	3	1.155	15	-.012	1	0	4	-.013	4	-.003	4
429		6	max	236.112	2	4.189	6	1.543	4	0	1	0	12	15
430		min	-344.785	3	.974	15	-.012	1	0	4	-.012	4	-.005	4
431		7	max	235.941	2	3.416	6	2.085	4	0	1	0	12	15
432		min	-344.912	3	.792	15	-.012	1	0	4	-.011	4	-.006	4
433		8	max	235.771	2	2.644	6	2.628	4	0	1	0	12	15
434		min	-345.04	3	.61	15	-.012	1	0	4	-.01	4	-.007	4
435		9	max	235.601	2	1.872	6	3.17	4	0	1	0	12	15
436		min	-345.168	3	.429	15	-.012	1	0	4	-.009	4	-.008	4
437		10	max	235.43	2	1.099	6	3.712	4	0	1	0	12	15
438		min	-345.296	3	.247	15	-.012	1	0	4	-.008	4	-.009	4
439		11	max	235.26	2	.383	2	4.254	4	0	1	0	12	15
440		min	-345.423	3	.048	12	-.012	1	0	4	-.006	4	-.009	4
441		12	max	235.09	2	-.116	15	4.796	4	0	1	0	12	15
442		min	-345.551	3	-.447	4	-.012	1	0	4	-.004	4	-.009	4
443		13	max	234.919	2	-.297	15	5.338	4	0	1	0	12	15
444		min	-345.679	3	-1.219	4	-.012	1	0	4	-.002	4	-.009	4
445		14	max	234.749	2	-.479	15	5.88	4	0	1	0	5	15
446		min	-345.807	3	-1.992	4	-.012	1	0	4	0	1	-.008	4
447		15	max	234.579	2	-.661	15	6.422	4	0	1	.003	4	15
448		min	-345.934	3	-2.764	4	-.012	1	0	4	0	1	-.007	4
449		16	max	234.408	2	-.842	15	6.965	4	0	1	.006	4	15
450		min	-346.062	3	-3.536	4	-.012	1	0	4	0	1	-.006	4
451		17	max	234.238	2	-1.024	15	7.507	4	0	1	.009	4	15
452		min	-346.19	3	-4.309	4	-.012	1	0	4	0	1	-.004	4
453		18	max	234.068	2	-1.205	15	8.049	4	0	1	.012	4	15
454		min	-346.318	3	-5.081	4	-.012	1	0	4	0	1	-.002	4
455		19	max	233.897	2	-1.387	15	8.591	4	0	1	.016	4	1
456		min	-346.445	3	-5.854	4	-.012	1	0	4	0	1	0	1
457	M12	1	max	1235.065	1	0	1	8.413	1	0	1	.008	4	1
458		min	-256.873	3	0	1	-236.067	4	0	1	0	1	0	1
459		2	max	1235.236	1	0	1	8.413	1	0	1	0	1	1
460		min	-256.745	3	0	1	-236.214	4	0	1	-.019	4	0	1
461		3	max	1235.406	1	0	1	8.413	1	0	1	.002	1	1
462		min	-256.617	3	0	1	-236.362	4	0	1	-.046	4	0	1
463		4	max	1235.576	1	0	1	8.413	1	0	1	.003	1	1
464		min	-256.49	3	0	1	-236.509	4	0	1	-.073	4	0	1
465		5	max	1235.747	1	0	1	8.413	1	0	1	.004	1	1
466		min	-256.362	3	0	1	-236.657	4	0	1	-.1	4	0	1
467		6	max	1235.917	1	0	1	8.413	1	0	1	.005	1	1
468		min	-256.234	3	0	1	-236.805	4	0	1	-.127	4	0	1
469		7	max	1236.087	1	0	1	8.413	1	0	1	.006	1	1
470		min	-256.106	3	0	1	-236.952	4	0	1	-.155	4	0	1
471		8	max	1236.258	1	0	1	8.413	1	0	1	.007	1	1
472		min	-255.979	3	0	1	-237.1	4	0	1	-.182	4	0	1
473		9	max	1236.428	1	0	1	8.413	1	0	1	.008	1	1
474		min	-255.851	3	0	1	-237.248	4	0	1	-.209	4	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1236.599	1	0	1	8.413	1	0	1	.009	1	0	1
476		min	-255.723	3	0	1	-237.395	4	0	1	-.236	4	0	1
477	11	max	1236.769	1	0	1	8.413	1	0	1	.01	1	0	1
478		min	-255.595	3	0	1	-237.543	4	0	1	-.264	4	0	1
479	12	max	1236.939	1	0	1	8.413	1	0	1	.01	1	0	1
480		min	-255.467	3	0	1	-237.691	4	0	1	-.291	4	0	1
481	13	max	1237.11	1	0	1	8.413	1	0	1	.011	1	0	1
482		min	-255.34	3	0	1	-237.838	4	0	1	-.318	4	0	1
483	14	max	1237.28	1	0	1	8.413	1	0	1	.012	1	0	1
484		min	-255.212	3	0	1	-237.986	4	0	1	-.346	4	0	1
485	15	max	1237.45	1	0	1	8.413	1	0	1	.013	1	0	1
486		min	-255.084	3	0	1	-238.133	4	0	1	-.373	4	0	1
487	16	max	1237.621	1	0	1	8.413	1	0	1	.014	1	0	1
488		min	-254.956	3	0	1	-238.281	4	0	1	-.4	4	0	1
489	17	max	1237.791	1	0	1	8.413	1	0	1	.015	1	0	1
490		min	-254.829	3	0	1	-238.429	4	0	1	-.428	4	0	1
491	18	max	1237.961	1	0	1	8.413	1	0	1	.016	1	0	1
492		min	-254.701	3	0	1	-238.576	4	0	1	-.455	4	0	1
493	19	max	1238.132	1	0	1	8.413	1	0	1	.017	1	0	1
494		min	-254.573	3	0	1	-238.724	4	0	1	-.482	4	0	1
495	M1	max	139.851	1	491.89	3	57.184	5	0	1	.211	1	0	3
496		min	-5.96	5	-511.684	1	-88.683	1	0	3	-.058	5	-.013	1
497	2	max	140.222	1	490.852	3	58.426	5	0	1	.164	1	.257	1
498		min	-5.787	5	-513.068	1	-88.683	1	0	3	-.028	5	-.259	3
499	3	max	202.618	3	566.327	1	-3.449	12	0	3	.117	1	.515	1
500		min	-131.334	2	-355.156	3	-87.644	1	0	1	.001	15	-.507	3
501	4	max	202.896	3	564.944	1	-3.449	12	0	3	.071	1	.216	1
502		min	-130.963	2	-356.194	3	-87.644	1	0	1	-.008	5	-.32	3
503	5	max	203.174	3	563.56	1	-3.449	12	0	3	.025	1	-.004	15
504		min	-130.593	2	-357.232	3	-87.644	1	0	1	-.017	5	-.131	3
505	6	max	203.452	3	562.176	1	-3.449	12	0	3	0	12	.057	3
506		min	-130.222	2	-358.27	3	-87.644	1	0	1	-.029	4	-.378	1
507	7	max	203.73	3	560.793	1	-3.449	12	0	3	-.003	12	.247	3
508		min	-129.851	2	-359.307	3	-87.644	1	0	1	-.068	1	-.675	1
509	8	max	204.008	3	559.409	1	-3.449	12	0	3	-.004	12	.437	3
510		min	-129.48	2	-360.345	3	-87.644	1	0	1	-.114	1	-.97	1
511	9	max	211.818	3	34.051	2	39.23	5	0	9	.067	1	.51	3
512		min	-73.651	2	.418	15	-128.438	1	0	3	-.119	5	-1.105	1
513	10	max	212.096	3	32.668	2	40.472	5	0	9	0	12	.497	3
514		min	-73.28	2	0	5	-128.438	1	0	3	-.099	4	-1.115	1
515	11	max	212.374	3	31.284	2	41.713	5	0	9	-.003	12	.484	3
516		min	-72.909	2	-1.732	4	-128.438	1	0	3	-.09	4	-1.123	1
517	12	max	220.146	3	237.06	3	127.621	5	0	1	.112	1	.421	3
518		min	-47.296	5	-595.224	1	-85.626	1	0	3	-.171	5	-.991	1
519	13	max	220.425	3	236.022	3	128.862	5	0	1	.067	1	.296	3
520		min	-47.123	5	-596.608	1	-85.626	1	0	3	-.103	5	-.677	1
521	14	max	220.703	3	234.984	3	130.103	5	0	1	.022	1	.172	3
522		min	-46.95	5	-597.991	1	-85.626	1	0	3	-.035	5	-.362	1
523	15	max	220.981	3	233.947	3	131.345	5	0	1	.034	5	.048	3
524		min	-46.777	5	-599.375	1	-85.626	1	0	3	-.023	1	-.046	1
525	16	max	221.259	3	232.909	3	132.586	5	0	1	.104	5	.271	1
526		min	-46.604	5	-600.758	1	-85.626	1	0	3	-.068	1	-.075	3
527	17	max	221.537	3	231.871	3	133.828	5	0	1	.174	5	.588	1
528		min	-46.431	5	-602.142	1	-85.626	1	0	3	-.114	1	-.197	3
529	18	max	13.025	5	582.415	1	-3.549	12	0	5	.144	5	.295	1
530		min	-140.39	1	-198.765	3	-110.459	4	0	1	-.163	1	-.098	3
531	19	max	13.198	5	581.031	1	-3.549	12	0	5	.097	5	.007	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-140.019	1	-199.803	3	-109.217	4	0	1	-.212	1	-.012	1
533	M5	max	301.779	1	1640.335	3	87.128	5	0	1	0	1	.027	1
534		min	9.244	12	-1730.799	1	0	1	0	4	-.132	4	0	3
535		max	302.15	1	1639.297	3	88.369	5	0	1	0	1	.941	1
536		min	9.429	12	-1732.183	1	0	1	0	4	-.086	4	-.866	3
537		max	650.743	3	1737.291	1	13.693	4	0	4	0	1	1.813	1
538		min	-496.927	1	-1144.631	3	0	1	0	1	-.04	4	-1.698	3
539		max	651.021	3	1735.907	1	14.935	4	0	4	0	1	.897	1
540		min	-496.556	1	-1145.669	3	0	1	0	1	-.033	4	-1.093	3
541		max	651.299	3	1734.524	1	16.176	4	0	4	0	1	.013	9
542		min	-496.186	1	-1146.706	3	0	1	0	1	-.025	4	-.488	3
543		max	651.577	3	1733.14	1	17.418	4	0	4	0	1	.117	3
544		min	-495.815	1	-1147.744	3	0	1	0	1	-.016	5	-.934	1
545		max	651.855	3	1731.757	1	18.659	4	0	4	0	1	.723	3
546		min	-495.444	1	-1148.782	3	0	1	0	1	-.008	5	-1.848	1
547		max	652.133	3	1730.373	1	19.901	4	0	4	.004	4	1.329	3
548		min	-495.073	1	-1149.82	3	0	1	0	1	0	1	-2.761	1
549		max	665.78	3	112.702	2	125.337	4	0	1	0	1	1.531	3
550		min	-367.266	2	.418	15	0	1	0	1	-.156	4	-3.122	1
551		max	666.058	3	111.318	2	126.579	4	0	1	0	1	1.483	3
552		min	-366.895	2	0	15	0	1	0	1	-.09	5	-3.154	1
553		max	666.336	3	109.934	2	127.82	4	0	1	0	1	1.436	3
554		min	-366.524	2	-1.587	6	0	1	0	1	-.024	5	-3.185	1
555		max	680.058	3	751.32	3	173.729	4	0	1	0	1	1.26	3
556		min	-252.168	2	-1850.62	1	0	1	0	4	-.237	4	-2.836	1
557		max	680.336	3	750.282	3	174.971	4	0	1	0	1	.864	3
558		min	-251.797	2	-1852.003	1	0	1	0	4	-.145	4	-1.859	1
559		max	680.614	3	749.244	3	176.212	4	0	1	0	1	.468	3
560		min	-251.426	2	-1853.387	1	0	1	0	4	-.052	4	-.882	1
561		max	680.892	3	748.206	3	177.454	4	0	1	.041	4	.143	2
562		min	-251.056	2	-1854.771	1	0	1	0	4	0	1	-.004	13
563		max	681.17	3	747.169	3	178.695	4	0	1	.135	4	1.076	1
564		min	-250.685	2	-1856.154	1	0	1	0	4	0	1	-.321	3
565		max	681.448	3	746.131	3	179.937	4	0	1	.229	4	2.055	1
566		min	-250.314	2	-1857.538	1	0	1	0	4	0	1	-.715	3
567		max	-9.576	12	1968.019	1	0	1	0	4	.225	4	1.063	1
568		min	-301.815	1	-680.541	3	-42.097	5	0	1	0	1	-.374	3
569		max	-9.391	12	1966.636	1	0	1	0	4	.204	4	.024	1
570		min	-301.444	1	-681.579	3	-40.855	5	0	1	0	1	-.014	3
571	M9	max	139.851	1	491.89	3	88.683	1	0	3	-.008	12	0	3
572		min	4.855	12	-511.684	1	3.497	12	0	1	-.211	1	-.013	1
573		max	140.222	1	490.852	3	88.683	1	0	3	-.006	12	.257	1
574		min	5.041	12	-513.068	1	3.497	12	0	1	-.164	1	-.259	3
575		max	202.618	3	566.327	1	87.644	1	0	1	-.005	12	.515	1
576		min	-131.334	2	-355.156	3	-10.388	5	0	3	-.117	1	-.507	3
577		max	202.896	3	564.944	1	87.644	1	0	1	-.003	12	.216	1
578		min	-130.963	2	-356.194	3	-9.146	5	0	3	-.071	1	-.32	3
579		max	203.174	3	563.56	1	87.644	1	0	1	-.001	12	-.004	15
580		min	-130.593	2	-357.232	3	-7.905	5	0	3	-.025	1	-.131	3
581		max	203.452	3	562.176	1	87.644	1	0	1	.021	1	.057	3
582		min	-130.222	2	-358.27	3	-6.663	5	0	3	-.023	5	-.378	1
583		max	203.73	3	560.793	1	87.644	1	0	1	.068	1	.247	3
584		min	-129.851	2	-359.307	3	-5.422	5	0	3	-.026	5	-.675	1
585		max	204.008	3	559.409	1	87.644	1	0	1	.114	1	.437	3
586		min	-129.48	2	-360.345	3	-4.181	5	0	3	-.029	5	-.97	1
587		max	211.818	3	34.051	2	128.438	1	0	3	-.003	12	.51	3
588		min	-73.651	2	.422	15	4.955	12	0	9	-.141	4	-1.105	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	212.096	3	32.668	2	128.438	1	0	3	0	1	.497	3
590		min	-73.28	2	.005	15	4.955	12	0	9	-.099	4	-1.115	1
591	11	max	212.374	3	31.284	2	128.438	1	0	3	.069	1	.484	3
592		min	-72.909	2	-1.693	6	4.955	12	0	9	-.07	5	-1.123	1
593	12	max	220.146	3	237.06	3	155.213	4	0	3	-.004	12	.421	3
594		min	-43.397	10	-595.224	1	3.232	12	0	1	-.206	4	-.991	1
595	13	max	220.425	3	236.022	3	156.455	4	0	3	-.003	12	.296	3
596		min	-43.088	10	-596.608	1	3.232	12	0	1	-.124	4	-.677	1
597	14	max	220.703	3	234.984	3	157.696	4	0	3	0	12	.172	3
598		min	-42.779	10	-597.991	1	3.232	12	0	1	-.041	4	-.362	1
599	15	max	220.981	3	233.947	3	158.938	4	0	3	.042	4	.048	3
600		min	-42.47	10	-599.375	1	3.232	12	0	1	0	12	-.046	1
601	16	max	221.259	3	232.909	3	160.179	4	0	3	.126	4	.271	1
602		min	-42.161	10	-600.758	1	3.232	12	0	1	.003	12	-.075	3
603	17	max	221.537	3	231.871	3	161.421	4	0	3	.211	4	.588	1
604		min	-41.852	10	-602.142	1	3.232	12	0	1	.004	12	-.197	3
605	18	max	-4.967	12	582.415	1	93.749	1	0	1	.196	4	.295	1
606		min	-140.39	1	-198.765	3	-81.428	5	0	3	.006	12	-.098	3
607	19	max	-4.782	12	581.031	1	93.749	1	0	1	.212	1	.007	3
608		min	-140.019	1	-199.803	3	-80.186	5	0	3	.008	12	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.112	1	.004	3	8.868e-3	1	NC	1	NC	1
2			min	-484	4	-.015	3	-.001	2	-1.112e-3	3	NC	1	NC	1
3		2	max	0	1	.235	3	.035	1	1.022e-2	1	NC	5	NC	2
4			min	-484	4	-.127	1	-.014	5	-1.161e-3	3	985.812	3	7361.326	1
5		3	max	0	1	.437	3	.084	1	1.157e-2	1	NC	5	NC	3
6			min	-484	4	-.315	1	-.016	5	-1.211e-3	3	544.985	3	3000.814	1
7		4	max	0	1	.559	3	.126	1	1.293e-2	1	NC	5	NC	3
8			min	-484	4	-.421	1	-.011	5	-1.261e-3	3	428.933	3	1981.426	1
9		5	max	0	1	.586	3	.148	1	1.428e-2	1	NC	5	NC	3
10			min	-484	4	-.429	1	-.002	5	-1.31e-3	3	409.328	3	1684.629	1
11		6	max	0	1	.521	3	.143	1	1.563e-2	1	NC	5	NC	3
12			min	-484	4	-.343	1	.005	15	-1.36e-3	3	459.001	3	1741.115	1
13		7	max	0	1	.383	3	.113	1	1.698e-2	1	NC	5	NC	3
14			min	-484	4	-.182	1	.007	10	-1.41e-3	3	618.644	3	2211.409	1
15		8	max	0	1	.208	3	.067	1	1.834e-2	1	NC	4	NC	2
16			min	-484	4	-.003	9	.002	10	-1.459e-3	3	1106.078	3	3796.684	1
17		9	max	0	1	.189	1	.02	1	1.969e-2	1	NC	4	NC	1
18			min	-484	4	.005	15	-.002	10	-1.509e-3	3	3166.82	1	NC	1
19		10	max	0	1	.268	1	.011	3	2.104e-2	1	NC	3	NC	1
20			min	-484	4	-.023	3	-.007	2	-1.559e-3	3	1578.104	1	NC	1
21		11	max	0	12	.189	1	.02	1	1.969e-2	1	NC	4	NC	1
22			min	-484	4	.005	15	-.011	5	-1.509e-3	3	3166.82	1	NC	1
23		12	max	0	12	.208	3	.067	1	1.834e-2	1	NC	4	NC	2
24			min	-484	4	-.003	9	-.011	5	-1.459e-3	3	1106.078	3	3796.684	1
25		13	max	0	12	.383	3	.113	1	1.698e-2	1	NC	5	NC	3
26			min	-484	4	-.182	1	-.004	5	-1.41e-3	3	618.644	3	2211.409	1
27		14	max	0	12	.521	3	.143	1	1.563e-2	1	NC	5	NC	3
28			min	-484	4	-.343	1	.004	15	-1.36e-3	3	459.001	3	1741.115	1
29		15	max	0	12	.586	3	.148	1	1.428e-2	1	NC	5	NC	3
30			min	-485	4	-.429	1	.009	12	-1.31e-3	3	409.328	3	1684.629	1
31		16	max	0	12	.559	3	.126	1	1.293e-2	1	NC	5	NC	3
32			min	-485	4	-.421	1	.008	12	-1.261e-3	3	428.933	3	1981.426	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.437	3	.084	1	1.157e-2	1	NC	5	NC	3
34		min	-485	4	-.315	1	.006	12	-1.211e-3	3	544.985	3	3000.814	1
35	18	max	0	12	.235	3	.035	1	1.022e-2	1	NC	5	NC	2
36		min	-485	4	-.127	1	.002	10	-1.161e-3	3	985.812	3	7361.326	1
37	19	max	0	12	.112	1	.004	3	8.868e-3	1	NC	1	NC	1
38		min	-485	4	-.015	3	-.001	2	-1.112e-3	3	NC	1	NC	1
39	M14	1	max	0	.15	3	.003	3	5.58e-3	1	NC	1	NC	1
40		min	-.387	4	-.361	1	-.001	10	-2.723e-3	3	NC	1	NC	1
41	2	max	0	1	.39	3	.025	1	6.713e-3	1	NC	5	NC	1
42		min	-.387	4	-.732	1	-.02	5	-3.323e-3	3	664.592	1	NC	1
43	3	max	0	1	.592	3	.068	1	7.845e-3	1	NC	15	NC	3
44		min	-.387	4	-1.049	1	-.024	5	-3.922e-3	3	357.862	1	3733.497	1
45	4	max	0	1	.731	3	.109	1	8.978e-3	1	NC	15	NC	3
46		min	-.387	4	-1.277	1	-.016	5	-4.522e-3	3	268.734	1	2310.03	1
47	5	max	0	1	.794	3	.132	1	1.011e-2	1	9468.875	15	NC	3
48		min	-.387	4	-1.397	1	-.002	5	-5.121e-3	3	237.516	1	1895.589	1
49	6	max	0	1	.781	3	.13	1	1.124e-2	1	9394.133	15	NC	3
50		min	-.387	4	-1.409	1	.009	12	-5.721e-3	3	234.739	1	1915.415	1
51	7	max	0	1	.706	3	.105	1	1.238e-2	1	NC	15	NC	3
52		min	-.387	4	-1.331	1	.006	10	-6.32e-3	3	253.669	1	2393.821	1
53	8	max	0	1	.596	3	.063	1	1.351e-2	1	NC	15	NC	2
54		min	-.387	4	-1.198	1	.002	10	-6.92e-3	3	294.009	1	4053.719	1
55	9	max	0	1	.49	3	.027	4	1.464e-2	1	NC	15	NC	1
56		min	-.387	4	-1.064	1	-.002	10	-7.52e-3	3	350.232	1	8946.295	4
57	10	max	0	1	.44	3	.01	3	1.577e-2	1	NC	5	NC	1
58		min	-.387	4	-1	1	-.006	2	-8.119e-3	3	385.325	1	NC	1
59	11	max	0	12	.49	3	.02	1	1.464e-2	1	NC	15	NC	1
60		min	-.387	4	-1.064	1	-.02	5	-7.52e-3	3	350.232	1	NC	1
61	12	max	0	12	.596	3	.063	1	1.351e-2	1	NC	15	NC	2
62		min	-.387	4	-1.198	1	-.023	5	-6.92e-3	3	294.009	1	4053.719	1
63	13	max	0	12	.706	3	.105	1	1.238e-2	1	NC	15	NC	3
64		min	-.387	4	-1.331	1	-.014	5	-6.32e-3	3	253.669	1	2393.821	1
65	14	max	0	12	.781	3	.13	1	1.124e-2	1	9393.797	15	NC	3
66		min	-.387	4	-1.409	1	0	15	-5.721e-3	3	234.739	1	1915.415	1
67	15	max	0	12	.794	3	.132	1	1.011e-2	1	9468.447	15	NC	3
68		min	-.387	4	-1.397	1	.008	12	-5.121e-3	3	237.516	1	1895.589	1
69	16	max	0	12	.731	3	.109	1	8.978e-3	1	NC	15	NC	3
70		min	-.387	4	-1.277	1	.007	12	-4.522e-3	3	268.734	1	2310.03	1
71	17	max	0	12	.592	3	.068	1	7.845e-3	1	NC	15	NC	3
72		min	-.387	4	-1.049	1	.005	10	-3.922e-3	3	357.862	1	3733.497	1
73	18	max	0	12	.39	3	.028	4	6.713e-3	1	NC	5	NC	1
74		min	-.387	4	-.732	1	0	10	-3.323e-3	3	664.592	1	8652.582	4
75	19	max	0	12	.15	3	.003	3	5.58e-3	1	NC	1	NC	1
76		min	-.387	4	-.361	1	-.001	10	-2.723e-3	3	NC	1	NC	1
77	M15	1	max	0	.154	3	.003	3	2.295e-3	3	NC	1	NC	1
78		min	-.326	4	-.361	1	0	10	-5.674e-3	1	NC	1	NC	1
79	2	max	0	12	.306	3	.025	1	2.803e-3	3	NC	5	NC	1
80		min	-.326	4	-.765	1	-.03	5	-6.83e-3	1	608.817	1	7921.174	5
81	3	max	0	12	.438	3	.068	1	3.31e-3	3	NC	15	NC	3
82		min	-.326	4	-1.11	1	-.037	5	-7.986e-3	1	328.542	1	3723.356	1
83	4	max	0	12	.534	3	.109	1	3.818e-3	3	NC	15	NC	3
84		min	-.326	4	-1.354	1	-.026	5	-9.142e-3	1	247.643	1	2305.079	1
85	5	max	0	12	.588	3	.132	1	4.325e-3	3	9476.982	15	NC	3
86		min	-.326	4	-1.479	1	-.007	5	-1.03e-2	1	220.144	1	1891.837	1
87	6	max	0	12	.598	3	.131	1	4.833e-3	3	9403.738	15	NC	3
88		min	-.326	4	-1.482	1	.008	12	-1.145e-2	1	219.465	1	1911.394	1
89	7	max	0	12	.573	3	.105	1	5.34e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.326	4	-1.385	1	.006	10	-1.261e-2	1	240.243	1	2387.505	1
91	8	max	0	12	.524	3	.063	1	5.848e-3	3	NC	15	NC	2
92		min	-.326	4	-1.228	1	.002	10	-1.377e-2	1	283.69	1	4035.954	1
93	9	max	0	12	.474	3	.036	4	6.355e-3	3	NC	15	NC	1
94		min	-.326	4	-1.072	1	-.002	10	-1.492e-2	1	345.846	1	6741.695	4
95	10	max	0	1	.45	3	.01	3	6.863e-3	3	NC	5	NC	1
96		min	-.326	4	-.999	1	-.006	2	-1.608e-2	1	385.824	1	NC	1
97	11	max	0	1	.474	3	.02	1	6.355e-3	3	NC	15	NC	1
98		min	-.326	4	-1.072	1	-.029	5	-1.492e-2	1	345.846	1	8513.987	5
99	12	max	0	1	.524	3	.063	1	5.848e-3	3	NC	15	NC	2
100		min	-.326	4	-1.228	1	-.034	5	-1.377e-2	1	283.69	1	4035.954	1
101	13	max	0	1	.573	3	.105	1	5.34e-3	3	NC	15	NC	3
102		min	-.326	4	-1.385	1	-.022	5	-1.261e-2	1	240.243	1	2387.505	1
103	14	max	0	1	.598	3	.131	1	4.833e-3	3	9403.492	15	NC	3
104		min	-.326	4	-1.482	1	-.002	5	-1.145e-2	1	219.465	1	1911.394	1
105	15	max	0	1	.588	3	.132	1	4.325e-3	3	9476.671	15	NC	3
106		min	-.326	4	-1.479	1	.008	12	-1.03e-2	1	220.144	1	1891.837	1
107	16	max	0	1	.534	3	.109	1	3.818e-3	3	NC	15	NC	3
108		min	-.326	4	-1.354	1	.006	12	-9.142e-3	1	247.643	1	2305.079	1
109	17	max	0	1	.438	3	.068	1	3.31e-3	3	NC	15	NC	3
110		min	-.326	4	-1.11	1	.005	12	-7.986e-3	1	328.542	1	3723.356	1
111	18	max	0	1	.306	3	.039	4	2.803e-3	3	NC	5	NC	1
112		min	-.326	4	-.765	1	.001	10	-6.83e-3	1	608.817	1	6360.204	4
113	19	max	0	1	.154	3	.003	3	2.295e-3	3	NC	1	NC	1
114		min	-.326	4	-.361	1	0	10	-5.674e-3	1	NC	1	NC	1
115	M16	1	max	0	12	.108	.003	3	3.986e-3	3	NC	1	NC	1
116		min	-.143	4	-.051	3	0	10	-8.345e-3	1	NC	1	NC	1
117	2	max	0	12	.039	3	.035	1	4.706e-3	3	NC	5	NC	2
118		min	-.144	4	-.165	1	-.023	5	-9.57e-3	1	899.48	1	7404.597	1
119	3	max	0	12	.109	3	.084	1	5.426e-3	3	NC	5	NC	3
120		min	-.144	4	-.383	1	-.028	5	-1.08e-2	1	501.164	1	3008.738	1
121	4	max	0	12	.147	3	.126	1	6.147e-3	3	NC	5	NC	3
122		min	-.144	4	-.506	1	-.021	5	-1.202e-2	1	400.264	1	1983.06	1
123	5	max	0	12	.146	3	.148	1	6.867e-3	3	NC	5	NC	3
124		min	-.144	4	-.519	1	-.007	5	-1.325e-2	1	392.019	1	1683.42	1
125	6	max	0	12	.109	3	.143	1	7.587e-3	3	NC	5	NC	3
126		min	-.144	4	-.425	1	.005	15	-1.447e-2	1	461.742	1	1736.625	1
127	7	max	0	12	.044	3	.114	1	8.307e-3	3	NC	5	NC	3
128		min	-.144	4	-.246	1	.007	10	-1.57e-2	1	695.349	1	2198.789	1
129	8	max	0	12	.004	4	.067	1	9.027e-3	3	NC	3	NC	2
130		min	-.144	4	-.053	2	.003	10	-1.692e-2	1	1806.671	2	3745.588	1
131	9	max	0	12	.17	1	.026	4	9.747e-3	3	NC	4	NC	1
132		min	-.144	4	-.104	3	-.001	10	-1.815e-2	1	4000.287	1	9457.215	4
133	10	max	0	1	.258	1	.008	3	1.047e-2	3	NC	5	NC	1
134		min	-.144	4	-.135	3	-.005	2	-1.937e-2	1	1647.714	1	NC	1
135	11	max	0	1	.17	1	.021	1	9.747e-3	3	NC	4	NC	1
136		min	-.144	4	-.104	3	-.018	5	-1.815e-2	1	4000.287	1	NC	1
137	12	max	0	1	.004	6	.067	1	9.027e-3	3	NC	3	NC	2
138		min	-.143	4	-.053	2	-.019	5	-1.692e-2	1	1806.671	2	3745.588	1
139	13	max	0	1	.044	3	.114	1	8.307e-3	3	NC	5	NC	3
140		min	-.143	4	-.246	1	-.009	5	-1.57e-2	1	695.349	1	2198.789	1
141	14	max	0	1	.109	3	.143	1	7.587e-3	3	NC	5	NC	3
142		min	-.143	4	-.425	1	.004	15	-1.447e-2	1	461.742	1	1736.625	1
143	15	max	0	1	.146	3	.148	1	6.867e-3	3	NC	5	NC	3
144		min	-.143	4	-.519	1	.008	12	-1.325e-2	1	392.019	1	1683.42	1
145	16	max	0	1	.147	3	.126	1	6.147e-3	3	NC	5	NC	3
146		min	-.143	4	-.506	1	.007	12	-1.202e-2	1	400.264	1	1983.06	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.109	3	.084	1	5.426e-3	3	NC	5	NC	3
148			min	-.143	4	-.383	1	.005	12	-1.08e-2	1	501.164	1	3008.738	1
149		18	max	0	1	.039	3	.035	1	4.706e-3	3	NC	5	NC	2
150			min	-.143	4	-.165	1	.002	10	-9.57e-3	1	899.48	1	7224.676	4
151		19	max	0	1	.108	1	.003	3	3.986e-3	3	NC	1	NC	1
152			min	-.143	4	-.051	3	0	10	-8.345e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.002	2	.007	1	1.213e-3	5	NC	1	NC	2
154			min	-.004	3	-.005	3	-.459	4	-1.756e-4	1	NC	1	104.32	4
155		2	max	.005	1	.002	2	.006	1	1.3e-3	5	NC	1	NC	2
156			min	-.004	3	-.005	3	-.421	4	-1.627e-4	1	NC	1	113.67	4
157		3	max	.005	1	.001	2	.006	1	1.387e-3	5	NC	1	NC	2
158			min	-.004	3	-.004	3	-.383	4	-1.498e-4	1	NC	1	124.789	4
159		4	max	.004	1	0	2	.005	1	1.473e-3	5	NC	1	NC	2
160			min	-.003	3	-.004	3	-.346	4	-1.369e-4	1	NC	1	138.138	4
161		5	max	.004	1	0	2	.005	1	1.56e-3	5	NC	1	NC	1
162			min	-.003	3	-.004	3	-.31	4	-1.24e-4	1	NC	1	154.349	4
163		6	max	.004	1	0	2	.004	1	1.647e-3	5	NC	1	NC	1
164			min	-.003	3	-.004	3	-.275	4	-1.111e-4	1	NC	1	174.296	4
165		7	max	.003	1	0	15	.004	1	1.733e-3	5	NC	1	NC	1
166			min	-.003	3	-.004	3	-.24	4	-9.825e-5	1	NC	1	199.222	4
167		8	max	.003	1	0	15	.003	1	1.82e-3	4	NC	1	NC	1
168			min	-.002	3	-.004	3	-.207	4	-8.536e-5	1	NC	1	230.944	4
169		9	max	.003	1	0	15	.003	1	1.911e-3	4	NC	1	NC	1
170			min	-.002	3	-.004	3	-.176	4	-7.247e-5	1	NC	1	272.202	4
171		10	max	.003	1	0	15	.002	1	2.003e-3	4	NC	1	NC	1
172			min	-.002	3	-.003	3	-.146	4	-5.958e-5	1	NC	1	327.288	4
173		11	max	.002	1	0	15	.002	1	2.094e-3	4	NC	1	NC	1
174			min	-.002	3	-.003	3	-.119	4	-4.669e-5	1	NC	1	403.247	4
175		12	max	.002	1	0	15	.001	1	2.186e-3	4	NC	1	NC	1
176			min	-.002	3	-.003	3	-.093	4	-3.38e-5	1	NC	1	512.307	4
177		13	max	.002	1	0	15	.001	1	2.277e-3	4	NC	1	NC	1
178			min	-.001	3	-.003	3	-.071	4	-2.091e-5	1	NC	1	677.265	4
179		14	max	.001	1	0	15	0	1	2.368e-3	4	NC	1	NC	1
180			min	-.001	3	-.002	3	-.051	4	-8.02e-6	1	NC	1	944.776	4
181		15	max	.001	1	0	15	0	1	2.46e-3	4	NC	1	NC	1
182			min	0	3	-.002	3	-.034	4	-3.208e-8	3	NC	1	1423.249	4
183		16	max	0	1	0	15	0	1	2.551e-3	4	NC	1	NC	1
184			min	0	3	-.001	6	-.02	4	5.074e-7	12	NC	1	2415.887	4
185		17	max	0	1	0	15	0	1	2.643e-3	4	NC	1	NC	1
186			min	0	3	-.001	6	-.009	4	1.019e-6	12	NC	1	5068.432	4
187		18	max	0	1	0	15	0	1	2.734e-3	4	NC	1	NC	1
188			min	0	3	0	6	-.003	4	1.53e-6	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.825e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	2.041e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-6.383e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.299e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.014	4	4.239e-6	1	NC	1	NC	1
194			min	0	2	-.001	6	0	12	-4.581e-6	5	NC	1	NC	1
195		3	max	0	3	0	15	.027	4	6.251e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	9.665e-7	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.039	4	1.253e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	1.769e-6	12	NC	1	NC	1
199		5	max	0	3	-.001	15	.051	4	1.88e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	2.571e-6	12	NC	1	8714.412	4
201		6	max	0	3	-.002	15	.063	4	2.508e-3	4	NC	1	NC	1
202			min	0	2	-.008	6	0	12	3.374e-6	12	NC	1	7842.706	4
203		7	max	.001	3	-.002	15	.073	4	3.135e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204		min	0	2	-.01	6	0	12	4.176e-6	12	9458.294	6	7448.54	4
205	8	max	.001	3	-.002	15	.084	4	3.763e-3	4	NC	1	NC	1
206		min	0	2	-.011	6	0	12	4.978e-6	12	8418.843	6	7386.754	4
207	9	max	.001	3	-.003	15	.094	4	4.39e-3	4	NC	1	NC	1
208		min	0	2	-.012	6	0	12	5.781e-6	12	7796.615	6	7607.453	5
209	10	max	.002	3	-.003	15	.103	4	5.018e-3	4	NC	2	NC	1
210		min	-.001	2	-.012	6	0	12	6.583e-6	12	7480.521	6	8124.71	5
211	11	max	.002	3	-.003	15	.112	4	5.645e-3	4	NC	2	NC	1
212		min	-.001	2	-.013	6	0	12	7.386e-6	12	7422.962	6	9016.007	5
213	12	max	.002	3	-.003	15	.121	4	6.273e-3	4	NC	2	NC	1
214		min	-.001	2	-.012	6	0	12	8.188e-6	12	7621.395	6	NC	1
215	13	max	.002	3	-.002	15	.13	4	6.9e-3	4	NC	1	NC	1
216		min	-.001	2	-.012	6	0	12	8.99e-6	12	8119.561	6	NC	1
217	14	max	.002	3	-.002	15	.138	4	7.528e-3	4	NC	1	NC	1
218		min	-.001	2	-.01	6	0	12	9.793e-6	12	9030.954	6	NC	1
219	15	max	.002	3	-.002	15	.147	4	8.155e-3	4	NC	1	NC	1
220		min	-.002	2	-.009	1	0	12	1.06e-5	12	NC	1	NC	1
221	16	max	.003	3	-.001	15	.155	4	8.783e-3	4	NC	1	NC	1
222		min	-.002	2	-.008	1	0	12	1.14e-5	12	NC	1	NC	1
223	17	max	.003	3	0	15	.164	4	9.411e-3	4	NC	1	NC	1
224		min	-.002	2	-.006	1	0	12	1.22e-5	12	NC	1	NC	1
225	18	max	.003	3	0	15	.174	4	1.004e-2	4	NC	1	NC	1
226		min	-.002	2	-.005	1	0	12	1.3e-5	12	NC	1	NC	1
227	19	max	.003	3	0	5	.184	4	1.067e-2	4	NC	1	NC	1
228		min	-.002	2	-.003	1	0	12	1.38e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.002	2	12	-3.995e-7	12	NC	1	NC	3
230		min	0	3	-.003	3	-.184	4	-9.493e-4	4	NC	1	134.945	4
231	2	max	.003	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
232		min	0	3	-.003	3	-.169	4	-9.493e-4	4	NC	1	146.926	4
233	3	max	.003	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
234		min	0	3	-.003	3	-.154	4	-9.493e-4	4	NC	1	161.173	4
235	4	max	.002	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
236		min	0	3	-.002	3	-.139	4	-9.493e-4	4	NC	1	178.278	4
237	5	max	.002	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
238		min	0	3	-.002	3	-.125	4	-9.493e-4	4	NC	1	199.043	4
239	6	max	.002	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
240		min	0	3	-.002	3	-.11	4	-9.493e-4	4	NC	1	224.581	4
241	7	max	.002	1	.001	2	0	12	-3.995e-7	12	NC	1	NC	2
242		min	0	3	-.002	3	-.097	4	-9.493e-4	4	NC	1	256.473	4
243	8	max	.002	1	0	2	0	12	-3.995e-7	12	NC	1	NC	2
244		min	0	3	-.002	3	-.084	4	-9.493e-4	4	NC	1	297.022	4
245	9	max	.002	1	0	2	0	12	-3.995e-7	12	NC	1	NC	2
246		min	0	3	-.002	3	-.071	4	-9.493e-4	4	NC	1	349.699	4
247	10	max	.001	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
248		min	0	3	-.001	3	-.059	4	-9.493e-4	4	NC	1	419.93	4
249	11	max	.001	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
250		min	0	3	-.001	3	-.048	4	-9.493e-4	4	NC	1	516.589	4
251	12	max	.001	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
252		min	0	3	-.001	3	-.038	4	-9.493e-4	4	NC	1	655.032	4
253	13	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
254		min	0	3	0	3	-.029	4	-9.493e-4	4	NC	1	863.768	4
255	14	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
256		min	0	3	0	3	-.021	4	-9.493e-4	4	NC	1	1200.803	4
257	15	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
258		min	0	3	0	3	-.014	4	-9.493e-4	4	NC	1	1799.882	4
259	16	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
260		min	0	3	0	3	-.008	4	-9.493e-4	4	NC	1	3030.836	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-9.493e-4	4	NC	1	6265.463	4
263		18	max	0	1	0	2	0	12	-3.995e-7	12	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-9.493e-4	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	-3.995e-7	12	NC	1	NC	1
266			min	0	1	0	1	0	1	-9.493e-4	4	NC	1	NC	1
267	M6	1	max	.017	1	.01	2	0	1	1.267e-3	4	NC	3	NC	1
268			min	-.013	3	-.015	3	-.463	4	0	1	4747.821	2	103.432	4
269		2	max	.016	1	.009	2	0	1	1.352e-3	4	NC	3	NC	1
270			min	-.012	3	-.014	3	-.425	4	0	1	5235.039	2	112.703	4
271		3	max	.015	1	.008	2	0	1	1.438e-3	4	NC	1	NC	1
272			min	-.012	3	-.013	3	-.387	4	0	1	5828.606	2	123.728	4
273		4	max	.014	1	.007	2	0	1	1.523e-3	4	NC	1	NC	1
274			min	-.011	3	-.012	3	-.349	4	0	1	6561.119	2	136.965	4
275		5	max	.013	1	.006	2	0	1	1.609e-3	4	NC	1	NC	1
276			min	-.01	3	-.012	3	-.313	4	0	1	7479.156	2	153.04	4
277		6	max	.012	1	.006	2	0	1	1.694e-3	4	NC	1	NC	1
278			min	-.009	3	-.011	3	-.277	4	0	1	8651.209	2	172.82	4
279		7	max	.011	1	.005	2	0	1	1.78e-3	4	NC	1	NC	1
280			min	-.009	3	-.01	3	-.242	4	0	1	NC	1	197.538	4
281		8	max	.01	1	.004	2	0	1	1.865e-3	4	NC	1	NC	1
282			min	-.008	3	-.009	3	-.209	4	0	1	NC	1	228.996	4
283		9	max	.009	1	.003	2	0	1	1.951e-3	4	NC	1	NC	1
284			min	-.007	3	-.009	3	-.177	4	0	1	NC	1	269.911	4
285		10	max	.008	1	.002	2	0	1	2.036e-3	4	NC	1	NC	1
286			min	-.007	3	-.008	3	-.147	4	0	1	NC	1	324.541	4
287		11	max	.007	1	.002	2	0	1	2.121e-3	4	NC	1	NC	1
288			min	-.006	3	-.007	3	-.12	4	0	1	NC	1	399.874	4
289		12	max	.006	1	.001	2	0	1	2.207e-3	4	NC	1	NC	1
290			min	-.005	3	-.006	3	-.094	4	0	1	NC	1	508.039	4
291		13	max	.006	1	0	2	0	1	2.292e-3	4	NC	1	NC	1
292			min	-.004	3	-.005	3	-.071	4	0	1	NC	1	671.655	4
293		14	max	.005	1	0	2	0	1	2.378e-3	4	NC	1	NC	1
294			min	-.004	3	-.004	3	-.051	4	0	1	NC	1	937.009	4
295		15	max	.004	1	0	2	0	1	2.463e-3	4	NC	1	NC	1
296			min	-.003	3	-.004	3	-.034	4	0	1	NC	1	1411.678	4
297		16	max	.003	1	0	2	0	1	2.549e-3	4	NC	1	NC	1
298			min	-.002	3	-.003	3	-.02	4	0	1	NC	1	2396.591	4
299		17	max	.002	1	0	2	0	1	2.634e-3	4	NC	1	NC	1
300			min	-.001	3	-.002	3	-.01	4	0	1	NC	1	5029.276	4
301		18	max	0	1	0	2	0	1	2.72e-3	4	NC	1	NC	1
302			min	0	3	0	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.805e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-6.231e-4	4	NC	1	NC	1
307		2	max	0	3	0	15	.014	4	0	1	NC	1	NC	1
308			min	0	2	-.002	3	0	1	-9.191e-6	5	NC	1	NC	1
309		3	max	.001	3	0	15	.027	4	6.069e-4	4	NC	1	NC	1
310			min	0	2	-.004	3	0	1	0	1	NC	1	NC	1
311		4	max	.002	3	-.001	15	.039	4	1.222e-3	4	NC	1	NC	1
312			min	-.001	2	-.005	3	0	1	0	1	NC	1	NC	1
313		5	max	.002	3	-.002	15	.051	4	1.837e-3	4	NC	1	NC	1
314			min	-.002	2	-.007	4	0	1	0	1	NC	1	8359.493	4
315		6	max	.003	3	-.002	15	.062	4	2.452e-3	4	NC	1	NC	1
316			min	-.002	2	-.009	4	0	1	0	1	NC	1	7499.671	4
317		7	max	.003	3	-.002	15	.073	4	3.067e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.003	2	-.01	4	0	1	0	1	9561.6	4	7096.68	4
319	8	max	.004	3	-.003	15	.083	4	3.682e-3	4	NC	1	NC	1
320		min	-.003	2	-.011	4	0	1	0	1	8503.383	4	7007.413	4
321	9	max	.004	3	-.003	15	.093	4	4.297e-3	4	NC	1	NC	1
322		min	-.004	2	-.012	4	0	1	0	1	7869.295	4	7183.674	4
323	10	max	.005	3	-.003	15	.102	4	4.912e-3	4	NC	1	NC	1
324		min	-.004	2	-.013	4	0	1	0	1	7545.805	4	7633.547	4
325	11	max	.005	3	-.003	15	.111	4	5.527e-3	4	NC	1	NC	1
326		min	-.005	2	-.013	4	0	1	0	1	7484.071	4	8417.327	4
327	12	max	.006	3	-.003	15	.119	4	6.142e-3	4	NC	1	NC	1
328		min	-.005	2	-.013	4	0	1	0	1	7680.99	4	9668.169	4
329	13	max	.006	3	-.003	15	.128	4	6.757e-3	4	NC	1	NC	1
330		min	-.006	2	-.012	4	0	1	0	1	8180.259	4	NC	1
331	14	max	.007	3	-.003	15	.136	4	7.372e-3	4	NC	1	NC	1
332		min	-.006	2	-.012	1	0	1	0	1	9095.899	4	NC	1
333	15	max	.007	3	-.002	15	.144	4	7.987e-3	4	NC	1	NC	1
334		min	-.007	2	-.011	1	0	1	0	1	NC	1	NC	1
335	16	max	.008	3	-.002	15	.153	4	8.602e-3	4	NC	1	NC	1
336		min	-.007	2	-.011	1	0	1	0	1	NC	1	NC	1
337	17	max	.008	3	-.001	15	.161	4	9.217e-3	4	NC	1	NC	1
338		min	-.008	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.009	3	0	15	.17	4	9.832e-3	4	NC	1	NC	1
340		min	-.008	2	-.009	1	0	1	0	1	NC	1	NC	1
341	19	max	.009	3	0	15	.18	4	1.045e-2	4	NC	1	NC	1
342		min	-.009	2	-.007	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.007	2	0	0	1	NC	1	NC	1
344		min	-.002	3	-.009	3	-.18	4	-9.711e-4	4	NC	1	137.948	4
345	2	max	.008	1	.007	2	0	1	0	1	NC	1	NC	1
346		min	-.002	3	-.009	3	-.165	4	-9.711e-4	4	NC	1	150.197	4
347	3	max	.008	1	.007	2	0	1	0	1	NC	1	NC	1
348		min	-.002	3	-.008	3	-.151	4	-9.711e-4	4	NC	1	164.764	4
349	4	max	.007	1	.006	2	0	1	0	1	NC	1	NC	1
350		min	-.002	3	-.008	3	-.136	4	-9.711e-4	4	NC	1	182.252	4
351	5	max	.007	1	.006	2	0	1	0	1	NC	1	NC	1
352		min	-.002	3	-.007	3	-.122	4	-9.711e-4	4	NC	1	203.482	4
353	6	max	.006	1	.005	2	0	1	0	1	NC	1	NC	1
354		min	-.002	3	-.007	3	-.108	4	-9.711e-4	4	NC	1	229.593	4
355	7	max	.006	1	.005	2	0	1	0	1	NC	1	NC	1
356		min	-.001	3	-.006	3	-.095	4	-9.711e-4	4	NC	1	262.199	4
357	8	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
358		min	-.001	3	-.006	3	-.082	4	-9.711e-4	4	NC	1	303.656	4
359	9	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
360		min	-.001	3	-.005	3	-.069	4	-9.711e-4	4	NC	1	357.514	4
361	10	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
362		min	-.001	3	-.005	3	-.058	4	-9.711e-4	4	NC	1	429.317	4
363	11	max	.004	1	.003	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.004	3	-.047	4	-9.711e-4	4	NC	1	528.141	4
365	12	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.004	3	-.037	4	-9.711e-4	4	NC	1	669.687	4
367	13	max	.003	1	.002	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.003	3	-.028	4	-9.711e-4	4	NC	1	883.099	4
369	14	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.003	3	-.02	4	-9.711e-4	4	NC	1	1227.686	4
371	15	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.002	3	-.013	4	-9.711e-4	4	NC	1	1840.191	4
373	16	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.008	4	-9.711e-4	4	NC	1	3098.739	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	0	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.001	3	-.004	4	-9.711e-4	4	NC	1	6405.897	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-9.711e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-9.711e-4	4	NC	1	NC	1
381	M10	1	max	.005	1	.002	2	0	12	1.268e-3	4	NC	1	NC	2
382			min	-.004	3	-.005	3	-.462	4	7.162e-6	12	NC	1	103.55	4
383		2	max	.005	1	.002	2	0	12	1.353e-3	4	NC	1	NC	2
384			min	-.004	3	-.005	3	-.424	4	6.651e-6	12	NC	1	112.832	4
385		3	max	.005	1	.001	2	0	12	1.438e-3	4	NC	1	NC	2
386			min	-.004	3	-.004	3	-.386	4	6.139e-6	12	NC	1	123.869	4
387		4	max	.004	1	0	2	0	12	1.523e-3	4	NC	1	NC	2
388			min	-.003	3	-.004	3	-.349	4	5.628e-6	12	NC	1	137.121	4
389		5	max	.004	1	0	2	0	12	1.608e-3	4	NC	1	NC	1
390			min	-.003	3	-.004	3	-.312	4	5.117e-6	12	NC	1	153.215	4
391		6	max	.004	1	0	2	0	12	1.693e-3	4	NC	1	NC	1
392			min	-.003	3	-.004	3	-.277	4	4.606e-6	12	NC	1	173.017	4
393		7	max	.003	1	0	2	0	12	1.778e-3	4	NC	1	NC	1
394			min	-.003	3	-.004	3	-.242	4	4.094e-6	12	NC	1	197.764	4
395		8	max	.003	1	0	2	0	12	1.863e-3	4	NC	1	NC	1
396			min	-.002	3	-.004	3	-.209	4	3.583e-6	12	NC	1	229.257	4
397		9	max	.003	1	0	10	0	12	1.948e-3	4	NC	1	NC	1
398			min	-.002	3	-.004	3	-.177	4	3.072e-6	12	NC	1	270.219	4
399		10	max	.003	1	0	10	0	12	2.033e-3	4	NC	1	NC	1
400			min	-.002	3	-.003	3	-.147	4	2.56e-6	12	NC	1	324.912	4
401		11	max	.002	1	0	15	0	12	2.118e-3	4	NC	1	NC	1
402			min	-.002	3	-.003	3	-.12	4	2.049e-6	12	NC	1	400.332	4
403		12	max	.002	1	0	15	0	12	2.202e-3	4	NC	1	NC	1
404			min	-.002	3	-.003	3	-.094	4	1.538e-6	12	NC	1	508.623	4
405		13	max	.002	1	0	15	0	12	2.287e-3	4	NC	1	NC	1
406			min	-.001	3	-.003	4	-.071	4	1.026e-6	12	NC	1	672.429	4
407		14	max	.001	1	0	15	0	12	2.372e-3	4	NC	1	NC	1
408			min	-.001	3	-.002	4	-.051	4	5.152e-7	12	NC	1	938.094	4
409		15	max	.001	1	0	15	0	12	2.457e-3	4	NC	1	NC	1
410			min	0	3	-.002	4	-.034	4	-4.871e-6	1	NC	1	1413.325	4
411		16	max	0	1	0	15	0	12	2.542e-3	4	NC	1	NC	1
412			min	0	3	-.002	4	-.02	4	-1.776e-5	1	NC	1	2399.421	4
413		17	max	0	1	0	15	0	12	2.627e-3	4	NC	1	NC	1
414			min	0	3	-.001	4	-.01	4	-3.065e-5	1	NC	1	5035.356	4
415		18	max	0	1	0	15	0	12	2.712e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.003	4	-4.354e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.797e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-5.643e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.749e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.211e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.014	4	-1.641e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-4.992e-6	5	NC	1	NC	1
423		3	max	0	3	0	15	.027	4	6.121e-4	4	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.597e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.039	4	1.229e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-4.77e-5	1	NC	1	NC	1
427		5	max	0	3	-.002	15	.051	4	1.845e-3	4	NC	1	NC	1
428			min	0	2	-.007	4	-.001	1	-6.943e-5	1	NC	1	8596.943	4
429		6	max	0	3	-.002	15	.062	4	2.462e-3	4	NC	1	NC	1
430			min	0	2	-.009	4	-.001	1	-9.116e-5	1	NC	1	7731.557	4
431		7	max	.001	3	-.003	15	.073	4	3.078e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432		min	0	2	-.01	4	-.002	1	-1.129e-4	1	9108.365	4	7336.789	4
433	8	max	.001	3	-.003	15	.083	4	3.695e-3	4	NC	1	NC	1
434		min	0	2	-.012	4	-.002	1	-1.346e-4	1	8131.383	4	7268.506	4
435	9	max	.001	3	-.003	15	.092	4	4.312e-3	4	NC	1	NC	1
436		min	0	2	-.013	4	-.002	1	-1.564e-4	1	7548.702	4	7480.733	4
437	10	max	.002	3	-.003	15	.102	4	4.928e-3	4	NC	2	NC	1
438		min	-.001	2	-.013	4	-.003	1	-1.781e-4	1	7257.253	4	7987.263	4
439	11	max	.002	3	-.003	15	.111	4	5.545e-3	4	NC	2	NC	1
440		min	-.001	2	-.013	4	-.003	1	-1.998e-4	1	7213.522	4	8859.68	4
441	12	max	.002	3	-.003	15	.119	4	6.161e-3	4	NC	2	NC	1
442		min	-.001	2	-.013	4	-.004	1	-2.216e-4	1	7416.773	4	NC	1
443	13	max	.002	3	-.003	15	.128	4	6.778e-3	4	NC	1	NC	1
444		min	-.001	2	-.012	4	-.004	1	-2.433e-4	1	7910.838	4	NC	1
445	14	max	.002	3	-.003	15	.136	4	7.395e-3	4	NC	1	NC	1
446		min	-.001	2	-.011	4	-.004	1	-2.65e-4	1	8807.343	4	NC	1
447	15	max	.002	3	-.002	15	.144	4	8.011e-3	4	NC	1	NC	1
448		min	-.002	2	-.01	4	-.005	1	-2.867e-4	1	NC	1	NC	1
449	16	max	.003	3	-.002	15	.153	4	8.628e-3	4	NC	1	NC	1
450		min	-.002	2	-.008	4	-.005	1	-3.085e-4	1	NC	1	NC	1
451	17	max	.003	3	-.001	15	.162	4	9.244e-3	4	NC	1	NC	1
452		min	-.002	2	-.006	1	-.006	1	-3.302e-4	1	NC	1	NC	1
453	18	max	.003	3	0	15	.171	4	9.861e-3	4	NC	1	NC	1
454		min	-.002	2	-.005	1	-.006	1	-3.519e-4	1	NC	1	NC	1
455	19	max	.003	3	0	12	.181	4	1.048e-2	4	NC	1	NC	1
456		min	-.002	2	-.003	1	-.006	1	-3.737e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.002	.006	1	1.369e-5	1	NC	1	NC	3
458		min	0	3	-.003	3	-.181	4	-9.423e-4	4	NC	1	137.388	4
459	2	max	.003	1	.001	2	.006	1	1.369e-5	1	NC	1	NC	2
460		min	0	3	-.003	3	-.166	4	-9.423e-4	4	NC	1	149.585	4
461	3	max	.003	1	.001	2	.005	1	1.369e-5	1	NC	1	NC	2
462		min	0	3	-.003	3	-.151	4	-9.423e-4	4	NC	1	164.089	4
463	4	max	.002	1	.001	2	.005	1	1.369e-5	1	NC	1	NC	2
464		min	0	3	-.002	3	-.137	4	-9.423e-4	4	NC	1	181.502	4
465	5	max	.002	1	.001	2	.004	1	1.369e-5	1	NC	1	NC	2
466		min	0	3	-.002	3	-.122	4	-9.423e-4	4	NC	1	202.641	4
467	6	max	.002	1	.001	2	.004	1	1.369e-5	1	NC	1	NC	2
468		min	0	3	-.002	3	-.108	4	-9.423e-4	4	NC	1	228.639	4
469	7	max	.002	1	.001	2	.003	1	1.369e-5	1	NC	1	NC	2
470		min	0	3	-.002	3	-.095	4	-9.423e-4	4	NC	1	261.105	4
471	8	max	.002	1	0	2	.003	1	1.369e-5	1	NC	1	NC	2
472		min	0	3	-.002	3	-.082	4	-9.423e-4	4	NC	1	302.385	4
473	9	max	.002	1	0	2	.002	1	1.369e-5	1	NC	1	NC	2
474		min	0	3	-.002	3	-.07	4	-9.423e-4	4	NC	1	356.011	4
475	10	max	.001	1	0	2	.002	1	1.369e-5	1	NC	1	NC	1
476		min	0	3	-.001	3	-.058	4	-9.423e-4	4	NC	1	427.506	4
477	11	max	.001	1	0	2	.002	1	1.369e-5	1	NC	1	NC	1
478		min	0	3	-.001	3	-.047	4	-9.423e-4	4	NC	1	525.906	4
479	12	max	.001	1	0	2	.001	1	1.369e-5	1	NC	1	NC	1
480		min	0	3	-.001	3	-.037	4	-9.423e-4	4	NC	1	666.843	4
481	13	max	0	1	0	2	.001	1	1.369e-5	1	NC	1	NC	1
482		min	0	3	0	3	-.028	4	-9.423e-4	4	NC	1	879.337	4
483	14	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
484		min	0	3	0	3	-.02	4	-9.423e-4	4	NC	1	1222.439	4
485	15	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
486		min	0	3	0	3	-.014	4	-9.423e-4	4	NC	1	1832.301	4
487	16	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
488		min	0	3	0	3	-.008	4	-9.423e-4	4	NC	1	3085.409	4



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-9.423e-4	4	NC	1	6378.232	4
491		18	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-9.423e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	1.369e-5	1	NC	1	NC	1
494			min	0	1	0	1	0	1	-9.423e-4	4	NC	1	NC	1
495	M1	1	max	.004	3	.112	1	.485	4	1.816e-2	1	NC	1	NC	1
496			min	-.001	2	-.015	3	0	12	-1.894e-2	3	NC	1	NC	1
497		2	max	.004	3	.055	1	.471	4	8.905e-3	4	NC	3	NC	1
498			min	-.001	2	-.007	3	-.005	1	-9.371e-3	3	2025.627	1	NC	1
499		3	max	.004	3	.005	3	.458	4	1.405e-2	4	NC	5	NC	1
500			min	-.001	2	-.006	1	-.007	1	-1.279e-4	1	968.432	1	8625.032	5
501		4	max	.004	3	.027	3	.446	4	1.245e-2	4	NC	5	NC	1
502			min	-.001	2	-.077	1	-.006	1	-3.427e-3	3	604.468	1	5845.637	5
503		5	max	.004	3	.056	3	.433	4	1.084e-2	4	NC	15	NC	1
504			min	-.001	2	-.153	1	-.004	1	-6.762e-3	3	432.138	1	4448.941	5
505		6	max	.004	3	.087	3	.421	4	1.541e-2	1	NC	15	NC	1
506			min	-.001	2	-.227	1	-.002	1	-1.01e-2	3	337.887	1	3632.01	5
507		7	max	.003	3	.117	3	.407	4	2.059e-2	1	9557.39	15	NC	1
508			min	-.001	10	-.293	1	0	12	-1.343e-2	3	282.583	1	3108.69	4
509		8	max	.003	3	.142	3	.393	4	2.577e-2	1	8489.959	15	NC	1
510			min	-.001	10	-.346	1	0	12	-1.677e-2	3	250.013	1	2758.45	4
511		9	max	.003	3	.158	3	.377	4	2.832e-2	1	7933.583	15	NC	1
512			min	-.001	10	-.379	1	0	1	-1.685e-2	3	233.111	1	2563.227	4
513		10	max	.003	3	.164	3	.359	4	2.912e-2	1	7764.188	15	NC	1
514			min	0	10	-.39	1	0	12	-1.478e-2	3	228.046	1	2511.656	4
515		11	max	.003	3	.161	3	.339	4	2.993e-2	1	7933.432	15	NC	1
516			min	0	10	-.379	1	0	12	-1.27e-2	3	233.369	1	2580.533	4
517		12	max	.003	3	.147	3	.317	4	2.82e-2	1	8489.611	15	NC	1
518			min	0	10	-.345	1	0	1	-1.061e-2	3	250.817	1	2787.893	5
519		13	max	.003	3	.125	3	.292	4	2.266e-2	1	9556.721	15	NC	1
520			min	0	10	-.291	1	0	1	-8.495e-3	3	284.571	1	3291.407	4
521		14	max	.003	3	.097	3	.264	4	1.712e-2	1	NC	15	NC	1
522			min	0	10	-.224	1	0	12	-6.378e-3	3	342.17	1	4305.05	4
523		15	max	.003	3	.065	3	.237	4	1.158e-2	1	NC	15	NC	1
524			min	0	10	-.149	1	0	12	-4.261e-3	3	440.996	1	6449.133	4
525		16	max	.003	3	.033	3	.209	4	9.293e-3	4	NC	5	NC	1
526			min	0	10	-.074	1	0	12	-2.144e-3	3	623.198	1	NC	1
527		17	max	.003	3	.002	3	.184	4	1.018e-2	4	NC	5	NC	1
528			min	0	10	-.004	1	0	12	-2.752e-5	3	1010.956	1	NC	1
529		18	max	.003	3	.055	1	.162	4	1.048e-2	1	NC	5	NC	1
530			min	0	10	-.025	3	0	12	-3.357e-3	3	2133.891	1	NC	1
531		19	max	.003	3	.108	1	.143	4	2.082e-2	1	NC	1	NC	1
532			min	0	10	-.051	3	0	1	-6.814e-3	3	NC	1	NC	1
533	M5	1	max	.011	3	.268	1	.484	4	0	1	NC	1	NC	1
534			min	-.007	2	-.023	3	0	1	-2.163e-6	4	NC	1	NC	1
535		2	max	.011	3	.133	1	.474	4	7.196e-3	4	NC	5	NC	1
536			min	-.007	2	-.012	3	0	1	0	1	847.693	1	NC	1
537		3	max	.011	3	.017	3	.462	4	1.417e-2	4	NC	15	NC	1
538			min	-.007	2	-.021	1	0	1	0	1	396.051	1	7211.62	4
539		4	max	.011	3	.078	3	.449	4	1.154e-2	4	9110.921	15	NC	1
540			min	-.007	2	-.209	1	0	1	0	1	240.144	1	5216.18	4
541		5	max	.011	3	.16	3	.436	4	8.92e-3	4	6377.323	15	NC	1
542			min	-.007	2	-.414	1	0	1	0	1	167.765	1	4191.448	4
543		6	max	.011	3	.252	3	.422	4	6.296e-3	4	4910.763	15	NC	1
544			min	-.007	2	-.619	1	0	1	0	1	128.966	1	3560.891	4
545		7	max	.011	3	.341	3	.407	4	3.671e-3	4	4063.65	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.006	2	-.805	1	0	1	0	1	106.569	1	3123.835	4
547		8	max	.01	3	.417	3	.393	4	1.046e-3	4	3571.019	15	NC	1
548			min	-.006	2	-.955	1	0	1	0	1	93.55	1	2791.369	4
549		9	max	.01	3	.465	3	.377	4	0	1	3318.355	15	NC	1
550			min	-.006	2	-1.049	1	0	1	-1.238e-6	5	86.88	1	2563.742	4
551		10	max	.01	3	.483	3	.359	4	0	1	3242.219	15	NC	1
552			min	-.006	2	-1.08	1	0	1	-1.171e-6	5	84.892	1	2529.548	4
553		11	max	.01	3	.471	3	.339	4	0	1	3318.407	15	NC	1
554			min	-.006	2	-1.048	1	0	1	-1.104e-6	5	86.984	1	2605.833	4
555		12	max	.01	3	.43	3	.318	4	7.33e-4	4	3571.146	15	NC	1
556			min	-.006	2	-.952	1	0	1	0	1	93.891	1	2751.208	4
557		13	max	.009	3	.364	3	.292	4	2.572e-3	4	4063.911	15	NC	1
558			min	-.006	2	-.8	1	0	1	0	1	107.455	1	3242.152	4
559		14	max	.009	3	.281	3	.264	4	4.411e-3	4	4911.28	15	NC	1
560			min	-.006	2	-.61	1	0	1	0	1	130.96	1	4423.371	4
561		15	max	.009	3	.188	3	.235	4	6.25e-3	4	6378.354	15	NC	1
562			min	-.005	2	-.403	1	0	1	0	1	172.086	1	7463.8	5
563		16	max	.009	3	.094	3	.206	4	8.089e-3	4	9113.089	15	NC	1
564			min	-.005	2	-.197	1	0	1	0	1	249.808	1	NC	1
565		17	max	.008	3	.006	3	.18	4	9.929e-3	4	NC	15	NC	1
566			min	-.005	2	-.013	1	0	1	0	1	419.509	1	NC	1
567		18	max	.008	3	.133	1	.16	4	5.042e-3	4	NC	5	NC	1
568			min	-.005	2	-.068	3	0	1	0	1	910.45	1	NC	1
569		19	max	.008	3	.258	1	.144	4	0	1	NC	1	NC	1
570			min	-.005	2	-.135	3	0	1	-8.907e-7	4	NC	1	NC	1
571	M9	1	max	.004	3	.112	1	.484	4	1.894e-2	3	NC	1	NC	1
572			min	-.001	2	-.015	3	0	1	-1.816e-2	1	NC	1	NC	1
573		2	max	.004	3	.055	1	.474	4	9.371e-3	3	NC	3	NC	1
574			min	-.001	2	-.007	3	0	12	-8.849e-3	1	2025.627	1	NC	1
575		3	max	.004	3	.005	3	.462	4	1.413e-2	4	NC	5	NC	1
576			min	-.001	2	-.006	1	0	12	-1.935e-5	10	968.432	1	7322.467	4
577		4	max	.004	3	.027	3	.449	4	1.108e-2	5	NC	5	NC	1
578			min	-.001	2	-.077	1	0	12	-5.051e-3	1	604.468	1	5254.076	4
579		5	max	.004	3	.056	3	.436	4	8.312e-3	5	NC	15	NC	1
580			min	-.001	2	-.153	1	0	12	-1.023e-2	1	432.138	1	4192.938	4
581		6	max	.004	3	.087	3	.422	4	1.01e-2	3	NC	15	NC	1
582			min	-.001	2	-.227	1	0	12	-1.541e-2	1	337.887	1	3545.417	4
583		7	max	.003	3	.117	3	.407	4	1.343e-2	3	9545.597	15	NC	1
584			min	-.001	10	-.293	1	0	1	-2.059e-2	1	282.583	1	3104.633	4
585		8	max	.003	3	.142	3	.393	4	1.677e-2	3	8479.716	15	NC	1
586			min	-.001	10	-.346	1	0	1	-2.577e-2	1	250.013	1	2776.477	5
587		9	max	.003	3	.158	3	.377	4	1.685e-2	3	7924.129	15	NC	1
588			min	-.001	10	-.379	1	0	12	-2.832e-2	1	233.111	1	2557.446	4
589		10	max	.003	3	.164	3	.359	4	1.478e-2	3	7754.964	15	NC	1
590			min	0	10	-.39	1	0	1	-2.912e-2	1	228.046	1	2512.515	4
591		11	max	.003	3	.161	3	.339	4	1.27e-2	3	7923.982	15	NC	1
592			min	0	10	-.379	1	0	1	-2.993e-2	1	233.369	1	2587.764	4
593		12	max	.003	3	.147	3	.317	4	1.061e-2	3	8479.439	15	NC	1
594			min	0	10	-.345	1	0	12	-2.82e-2	1	250.817	1	2769.971	4
595		13	max	.003	3	.125	3	.292	4	8.495e-3	3	9545.164	15	NC	1
596			min	0	10	-.291	1	0	12	-2.266e-2	1	284.571	1	3292.108	4
597		14	max	.003	3	.097	3	.264	4	6.378e-3	3	NC	15	NC	1
598			min	0	10	-.224	1	-.002	1	-1.712e-2	1	342.17	1	4405.941	5
599		15	max	.003	3	.065	3	.235	4	5.818e-3	5	NC	15	NC	1
600			min	0	10	-.149	1	-.004	1	-1.158e-2	1	440.996	1	6965.446	5
601		16	max	.003	3	.033	3	.206	4	7.849e-3	5	NC	5	NC	1
602			min	0	10	-.074	1	-.006	1	-6.035e-3	1	623.198	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.003	3	.002	3	.181	4	9.952e-3	4	NC	5	NC	1
604		min	0	10	-.004	1	-.007	1	-4.941e-4	1	1010.956	1	NC	1
605	18	max	.003	3	.055	1	.16	4	4.663e-3	5	NC	5	NC	1
606		min	0	10	-.025	3	-.005	1	-1.048e-2	1	2133.891	1	NC	1
607	19	max	.003	3	.108	1	.143	4	6.814e-3	3	NC	1	NC	1
608		min	0	10	-.051	3	0	12	-2.082e-2	1	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298





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Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

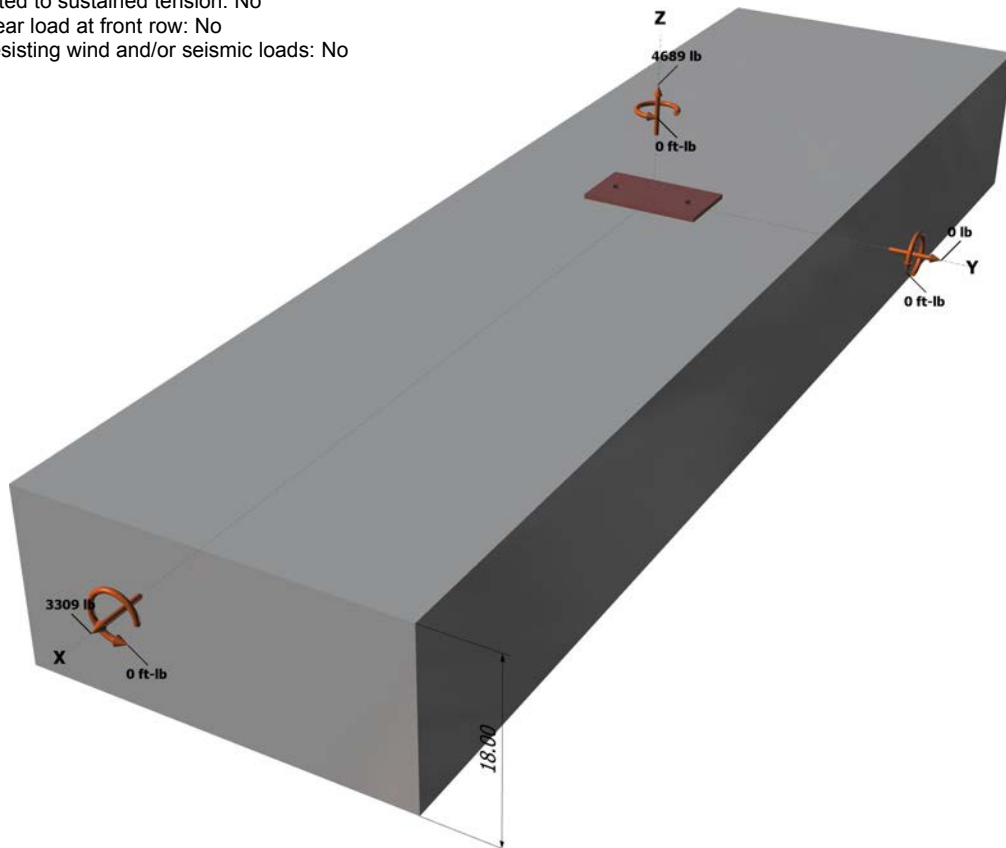
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

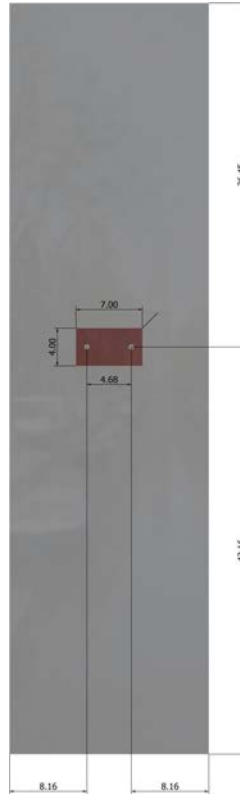
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Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

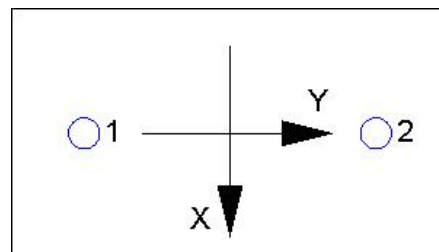
Company:	Schletter, Inc.	Date:	11/17/2015
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Address:			
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E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.