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1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	140 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 30.77 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.806 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	7%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

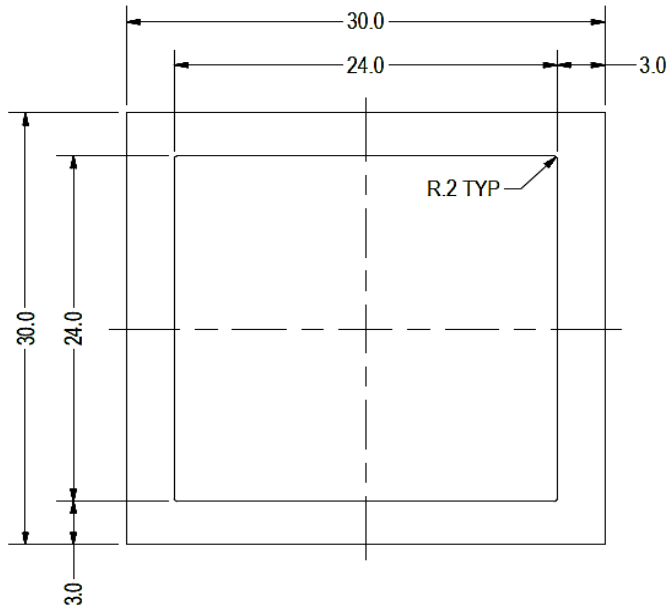
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.595 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	16%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

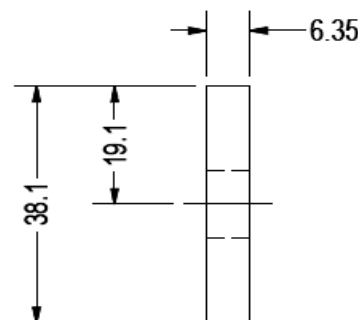
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.666 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	13%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.095 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 38 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

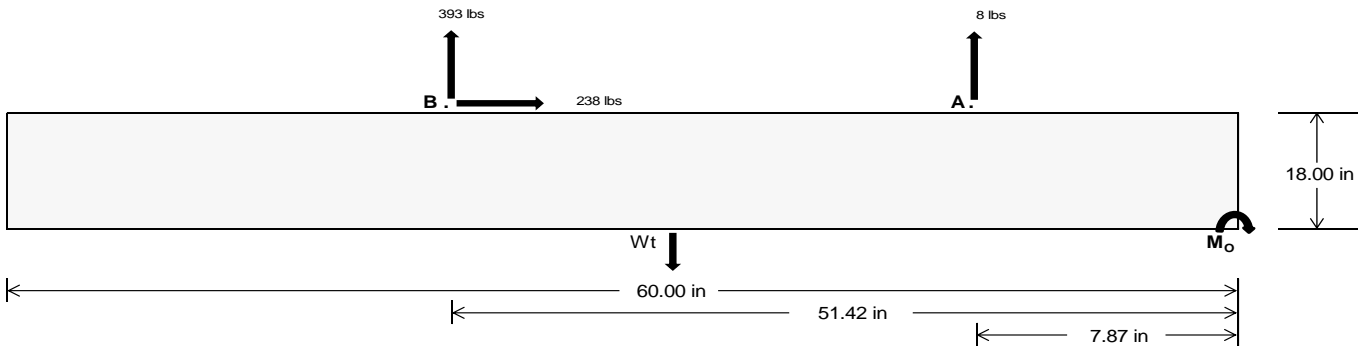
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	38.39	1708.01	k
Compressive Load =	1047.57	1154.93	k
Lateral Load =	1.70	1031.85	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24568.8$ in-lbs
Resisting Force Required = 818.96 lbs
S.F. = 1.67
Weight Required = 1364.93 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 238.02 lbs
Friction = 0.4
Weight Required = 595.05 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 238.02 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	361 lbs	361 lbs	361 lbs	361 lbs	372 lbs	372 lbs	372 lbs	372 lbs	517 lbs	517 lbs	517 lbs	517 lbs	-15 lbs	-15 lbs	-15 lbs	-15 lbs
F_B	246 lbs	246 lbs	246 lbs	246 lbs	486 lbs	486 lbs	486 lbs	486 lbs	526 lbs	526 lbs	526 lbs	526 lbs	-787 lbs	-787 lbs	-787 lbs	-787 lbs
F_V	36 lbs	36 lbs	36 lbs	36 lbs	428 lbs	428 lbs	428 lbs	428 lbs	345 lbs	345 lbs	345 lbs	345 lbs	-476 lbs	-476 lbs	-476 lbs	-476 lbs
P_{total}	2419 lbs	2510 lbs	2601 lbs	2691 lbs	2671 lbs	2762 lbs	2852 lbs	2943 lbs	2855 lbs	2946 lbs	3036 lbs	3127 lbs	286 lbs	340 lbs	394 lbs	449 lbs
M	280 lbs-ft	280 lbs-ft	280 lbs-ft	280 lbs-ft	461 lbs-ft	461 lbs-ft	461 lbs-ft	461 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	662 lbs-ft	662 lbs-ft	662 lbs-ft	662 lbs-ft
e	0.12 ft	0.11 ft	0.11 ft	0.10 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.18 ft	0.18 ft	0.17 ft	2.32 ft	1.95 ft	1.68 ft	1.48 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	250.0 psf	248.5 psf	247.1 psf	245.8 psf	254.2 psf	252.4 psf	250.8 psf	249.4 psf	266.0 psf	263.7 psf	261.6 psf	259.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	330.6 psf	325.2 psf	320.3 psf	315.9 psf	386.9 psf	378.8 psf	371.5 psf	364.8 psf	419.2 psf	409.6 psf	400.9 psf	392.9 psf	627.1 psf	234.4 psf	174.7 psf	152.4 psf

Maximum Bearing Pressure = 627 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

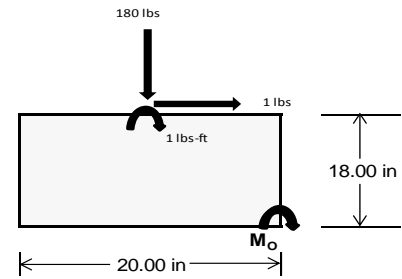
Overturning Check

$M_o = 147.9 \text{ ft-lbs}$
 Resisting Force Required = 177.52 lbs
 S.F. = 1.67
 Weight Required = 295.87 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	57 lbs	140 lbs	54 lbs	180 lbs	504 lbs	177 lbs	17 lbs	41 lbs	16 lbs
F_v	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2301 lbs	2384 lbs	2298 lbs	2316 lbs	2640 lbs	2313 lbs	673 lbs	697 lbs	672 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.28 ft	1.67 ft	1.67 ft	1.66 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft	1.67 ft
f_{min}	276.0 sqft	286.0 sqft	275.7 sqft	277.0 sqft	316.5 sqft	277.3 sqft	80.7 sqft	83.6 sqft	80.6 sqft
f_{max}	276.3 psf	286.2 psf	275.8 psf	278.9 psf	317.1 psf	277.8 psf	80.8 psf	83.7 psf	80.7 psf



Maximum Bearing Pressure = 317 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

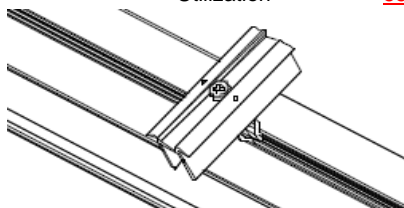
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

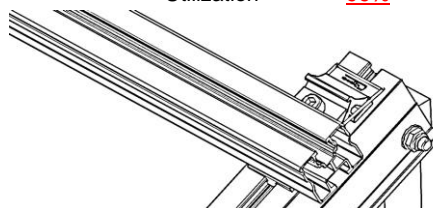
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.665 k
Allowable Uplift =	1.214 k
Utilization =	<u>55%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.067 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

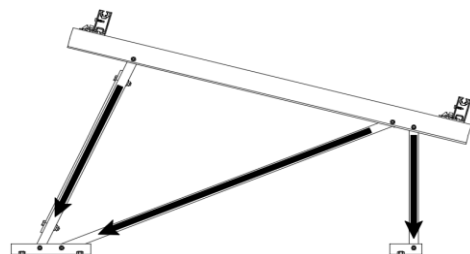
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.806 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>

Diagonal Strut

Maximum Axial Load =	0.595 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>



Rear Strut

Maximum Axial Load =	1.092 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.095 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.009 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.25 \\ &21.9891 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.7 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.25 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.7 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

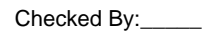
Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	217.196	2	275.976	2	0	10	0	15	0	1	0	1
2		min	-259.414	3	-410.47	3	-.134	3	0	3	0	1	0	1
3	N7	max	.002	3	295.208	1	-.026	10	0	15	0	1	0	1
4		min	-.13	2	3.536	12	-.633	1	-.001	1	0	1	0	1
5	N15	max	0	15	805.821	1	.22	9	0	1	0	1	0	1
6		min	-1.307	2	-29.53	3	-.616	3	0	3	0	1	0	1
7	N16	max	725.737	2	888.406	2	0	2	0	9	0	1	0	1
8		min	-793.732	3	-1313.854	3	-75.448	3	0	3	0	1	0	1
9	N23	max	.002	3	295.252	1	1.125	1	.002	1	0	1	0	1
10		min	-.13	2	3.922	12	.026	10	0	10	0	1	0	1
11	N24	max	217.196	2	278.574	2	76.07	3	0	1	0	1	0	1
12		min	-259.856	3	-409.334	3	0	10	0	3	0	1	0	1
13	Totals:	max	1158.561	2	2664.71	2	0	2						
14		min	-1313.042	3	-2154.67	3	0	9						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	210.341	1	.655	4	.171	1	0	10	0	15	0	1
2			min	-352.696	3	.154	15	-.065	3	0	1	0	1	0	1
3		2	max	210.466	1	.604	4	.171	1	0	10	0	15	0	15
4			min	-352.602	3	.142	15	-.065	3	0	1	0	1	0	4
5		3	max	210.592	1	.553	4	.171	1	0	10	0	15	0	15
6			min	-352.507	3	.13	15	-.065	3	0	1	0	3	0	4
7		4	max	210.718	1	.502	4	.171	1	0	10	0	9	0	15
8			min	-352.413	3	.118	15	-.065	3	0	1	0	3	0	4
9		5	max	210.844	1	.451	4	.171	1	0	10	0	9	0	15
10			min	-352.319	3	.106	15	-.065	3	0	1	0	3	0	4
11		6	max	210.97	1	.4	4	.171	1	0	10	0	1	0	15
12			min	-352.224	3	.094	15	-.065	3	0	1	0	3	0	4
13		7	max	211.096	1	.349	4	.171	1	0	10	0	1	0	15
14			min	-352.13	3	.082	15	-.065	3	0	1	0	3	0	4
15		8	max	211.222	1	.297	4	.171	1	0	10	0	1	0	15
16			min	-352.035	3	.07	15	-.065	3	0	1	0	3	0	4
17		9	max	211.348	1	.246	4	.171	1	0	10	0	1	0	15
18			min	-351.941	3	.058	15	-.065	3	0	1	0	3	0	4
19		10	max	211.473	1	.195	4	.171	1	0	10	0	1	0	15
20			min	-351.847	3	.046	15	-.065	3	0	1	0	3	0	4
21		11	max	211.599	1	.144	4	.171	1	0	10	0	1	0	15
22			min	-351.752	3	.034	15	-.065	3	0	1	0	3	0	4
23		12	max	211.725	1	.102	2	.171	1	0	10	0	1	0	15
24			min	-351.658	3	.014	12	-.065	3	0	1	0	3	0	4
25		13	max	211.851	1	.062	2	.171	1	0	10	0	1	0	15
26			min	-351.563	3	-.012	3	-.065	3	0	1	0	3	0	4
27		14	max	211.977	1	.022	2	.171	1	0	10	0	1	0	15
28			min	-351.469	3	-.042	3	-.065	3	0	1	0	3	0	4
29		15	max	212.103	1	-.014	15	.171	1	0	10	0	1	0	15
30			min	-351.375	3	-.072	3	-.065	3	0	1	0	3	0	4
31		16	max	212.229	1	-.026	15	.171	1	0	10	0	1	0	15
32			min	-351.28	3	-.112	4	-.065	3	0	1	0	3	0	4
33		17	max	212.354	1	-.038	15	.171	1	0	10	0	1	0	15
34			min	-351.186	3	-.163	4	-.065	3	0	1	0	3	0	4
35		18	max	212.48	1	-.05	15	.171	1	0	10	0	1	0	15
36			min	-351.091	3	-.214	4	-.065	3	0	1	0	3	0	4
37		19	max	212.606	1	-.062	15	.171	1	0	10	0	1	0	15





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	294.626	1	0	1	-.027	15	0	1	0	15	0	1
96		min	3.245	12	0	1	-.666	1	0	1	0	1	0	1
97	11	max	294.69	1	0	1	-.027	15	0	1	0	15	0	1
98		min	3.277	12	0	1	-.666	1	0	1	0	1	0	1
99	12	max	294.755	1	0	1	-.027	15	0	1	0	15	0	1
100		min	3.309	12	0	1	-.666	1	0	1	0	1	0	1
101	13	max	294.82	1	0	1	-.027	15	0	1	0	15	0	1
102		min	3.342	12	0	1	-.666	1	0	1	0	1	0	1
103	14	max	294.884	1	0	1	-.027	15	0	1	0	15	0	1
104		min	3.374	12	0	1	-.666	1	0	1	0	1	0	1
105	15	max	294.949	1	0	1	-.027	15	0	1	0	15	0	1
106		min	3.406	12	0	1	-.666	1	0	1	0	1	0	1
107	16	max	295.014	1	0	1	-.027	15	0	1	0	15	0	1
108		min	3.439	12	0	1	-.666	1	0	1	0	1	0	1
109	17	max	295.078	1	0	1	-.027	15	0	1	0	15	0	1
110		min	3.471	12	0	1	-.666	1	0	1	0	1	0	1
111	18	max	295.143	1	0	1	-.027	15	0	1	0	15	0	1
112		min	3.503	12	0	1	-.666	1	0	1	-.001	1	0	1
113	19	max	295.208	1	0	1	-.027	15	0	1	0	15	0	1
114		min	3.536	12	0	1	-.666	1	0	1	-.001	1	0	1
115	M6	1	max	663.302	1	.656	.037	9	0	3	0	3	0	1
116		min	-1092.007	3	.154	15	-.237	3	0	2	0	2	0	1
117	2	max	663.428	1	.605	4	.037	9	0	3	0	3	0	15
118		min	-1091.913	3	.142	15	-.237	3	0	2	0	2	0	4
119	3	max	663.554	1	.554	4	.037	9	0	3	0	3	0	15
120		min	-1091.818	3	.13	15	-.237	3	0	2	0	2	0	4
121	4	max	663.68	1	.503	4	.037	9	0	3	0	3	0	15
122		min	-1091.724	3	.118	15	-.237	3	0	2	0	2	0	4
123	5	max	663.806	1	.451	4	.037	9	0	3	0	3	0	15
124		min	-1091.63	3	.106	15	-.237	3	0	2	0	2	0	4
125	6	max	663.932	1	.407	2	.037	9	0	3	0	9	0	15
126		min	-1091.535	3	.088	12	-.237	3	0	2	0	2	0	4
127	7	max	664.057	1	.367	2	.037	9	0	3	0	9	0	15
128		min	-1091.441	3	.068	12	-.237	3	0	2	0	3	0	4
129	8	max	664.183	1	.327	2	.037	9	0	3	0	9	0	15
130		min	-1091.346	3	.048	12	-.237	3	0	2	0	3	0	4
131	9	max	664.309	1	.287	2	.037	9	0	3	0	9	0	15
132		min	-1091.252	3	.028	12	-.237	3	0	2	0	3	0	4
133	10	max	664.435	1	.247	2	.037	9	0	3	0	9	0	12
134		min	-1091.158	3	.001	3	-.237	3	0	2	0	3	0	2
135	11	max	664.561	1	.207	2	.037	9	0	3	0	9	0	12
136		min	-1091.063	3	-.029	3	-.237	3	0	2	0	3	0	2
137	12	max	664.687	1	.168	2	.037	9	0	3	0	9	0	12
138		min	-1090.969	3	-.059	3	-.237	3	0	2	0	3	0	2
139	13	max	664.813	1	.128	2	.037	9	0	3	0	9	0	12
140		min	-1090.874	3	-.088	3	-.237	3	0	2	0	3	0	2
141	14	max	664.939	1	.088	2	.037	9	0	3	0	9	0	12
142		min	-1090.78	3	-.118	3	-.237	3	0	2	0	3	0	2
143	15	max	665.064	1	.048	2	.037	9	0	3	0	9	0	12
144		min	-1090.686	3	-.148	3	-.237	3	0	2	0	3	0	2
145	16	max	665.19	1	.008	2	.037	9	0	3	0	9	0	12
146		min	-1090.591	3	-.178	3	-.237	3	0	2	0	3	0	2
147	17	max	665.316	1	-.032	2	.037	9	0	3	0	9	0	12
148		min	-1090.497	3	-.208	3	-.237	3	0	2	0	3	0	2
149	18	max	665.442	1	-.05	15	.037	9	0	3	0	9	0	3
150		min	-1090.402	3	-.238	3	-.237	3	0	2	0	3	0	2
151	19	max	665.568	1	-.062	15	.037	9	0	3	0	9	0	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1090.308	3	-.268	3	-.237	3	0	2	0	3	0	2
153	M7	1	max	595.055	2	1.761	4	.031	3	0	1	0	1	2
154		min	-499.921	3	.414	15	-.01	1	0	3	0	3	0	3
155		2	max	594.986	2	1.584	4	.031	3	0	1	0	1	2
156		min	-499.973	3	.372	15	-.01	1	0	3	0	3	0	3
157		3	max	594.917	2	1.407	4	.031	3	0	1	0	1	2
158		min	-500.025	3	.331	15	-.01	1	0	3	0	3	0	3
159		4	max	594.847	2	1.23	4	.031	3	0	1	0	1	2
160		min	-500.077	3	.289	15	-.01	1	0	3	0	3	0	3
161		5	max	594.778	2	1.054	4	.031	3	0	1	0	1	15
162		min	-500.129	3	.248	15	-.01	1	0	3	0	3	0	3
163		6	max	594.709	2	.877	4	.031	3	0	1	0	1	15
164		min	-500.181	3	.206	15	-.01	1	0	3	0	3	0	4
165		7	max	594.639	2	.7	4	.031	3	0	1	0	1	15
166		min	-500.233	3	.165	15	-.01	1	0	3	0	3	0	4
167		8	max	594.57	2	.523	4	.031	3	0	1	0	1	15
168		min	-500.285	3	.123	15	-.01	1	0	3	0	3	-.001	4
169		9	max	594.501	2	.349	2	.031	3	0	1	0	1	15
170		min	-500.337	3	.077	12	-.01	1	0	3	0	3	-.001	4
171		10	max	594.431	2	.212	2	.031	3	0	1	0	1	15
172		min	-500.389	3	.003	3	-.01	1	0	3	0	3	-.001	4
173		11	max	594.362	2	.074	2	.031	3	0	1	0	1	15
174		min	-500.441	3	-.101	3	-.01	1	0	3	0	3	-.001	4
175		12	max	594.293	2	-.043	15	.031	3	0	1	0	1	15
176		min	-500.493	3	-.204	3	-.01	1	0	3	0	3	-.001	4
177		13	max	594.223	2	-.085	15	.031	3	0	1	0	1	15
178		min	-500.545	3	-.361	4	-.01	1	0	3	0	3	-.001	4
179		14	max	594.154	2	-.126	15	.031	3	0	1	0	1	15
180		min	-500.597	3	-.538	4	-.01	1	0	3	0	3	-.001	4
181		15	max	594.085	2	-.168	15	.031	3	0	1	0	1	15
182		min	-500.649	3	-.715	4	-.01	1	0	3	0	3	0	4
183		16	max	594.015	2	-.21	15	.031	3	0	1	0	1	15
184		min	-500.701	3	-.892	4	-.01	1	0	3	0	3	0	4
185		17	max	593.946	2	-.251	15	.031	3	0	1	0	1	15
186		min	-500.753	3	-1.068	4	-.01	1	0	3	0	3	0	4
187		18	max	593.877	2	-.293	15	.031	3	0	1	0	1	15
188		min	-500.805	3	-1.245	4	-.01	1	0	3	0	3	0	4
189		19	max	593.807	2	-.334	15	.031	3	0	1	0	1	1
190		min	-500.857	3	-1.422	4	-.01	1	0	3	0	3	0	1
191	M8	1	max	804.656	1	0	1	.253	1	0	1	0	2	1
192		min	-30.404	3	0	1	-.613	3	0	1	0	3	0	1
193		2	max	804.721	1	0	1	.253	1	0	1	0	1	1
194		min	-30.355	3	0	1	-.613	3	0	1	0	3	0	1
195		3	max	804.786	1	0	1	.253	1	0	1	0	1	1
196		min	-30.307	3	0	1	-.613	3	0	1	0	3	0	1
197		4	max	804.851	1	0	1	.253	1	0	1	0	1	1
198		min	-30.258	3	0	1	-.613	3	0	1	0	3	0	1
199		5	max	804.915	1	0	1	.253	1	0	1	0	1	1
200		min	-30.21	3	0	1	-.613	3	0	1	0	3	0	1
201		6	max	804.98	1	0	1	.253	1	0	1	0	1	1
202		min	-30.161	3	0	1	-.613	3	0	1	0	3	0	1
203		7	max	805.045	1	0	1	.253	1	0	1	0	1	1
204		min	-30.113	3	0	1	-.613	3	0	1	0	3	0	1
205		8	max	805.109	1	0	1	.253	1	0	1	0	1	1
206		min	-30.064	3	0	1	-.613	3	0	1	0	3	0	1
207		9	max	805.174	1	0	1	.253	1	0	1	0	1	1
208		min	-30.016	3	0	1	-.613	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	805.239	1	0	1	.253	1	0	1	0	1	0	1
210			min	-29.967	3	0	1	-.613	3	0	1	0	3	0	1
211		11	max	805.304	1	0	1	.253	1	0	1	0	1	0	1
212			min	-29.918	3	0	1	-.613	3	0	1	0	3	0	1
213		12	max	805.368	1	0	1	.253	1	0	1	0	1	0	1
214			min	-29.87	3	0	1	-.613	3	0	1	0	3	0	1
215		13	max	805.433	1	0	1	.253	1	0	1	0	1	0	1
216			min	-29.821	3	0	1	-.613	3	0	1	0	3	0	1
217		14	max	805.498	1	0	1	.253	1	0	1	0	1	0	1
218			min	-29.773	3	0	1	-.613	3	0	1	0	3	0	1
219		15	max	805.562	1	0	1	.253	1	0	1	0	1	0	1
220			min	-29.724	3	0	1	-.613	3	0	1	0	3	0	1
221		16	max	805.627	1	0	1	.253	1	0	1	0	1	0	1
222			min	-29.676	3	0	1	-.613	3	0	1	0	3	0	1
223		17	max	805.692	1	0	1	.253	1	0	1	0	1	0	1
224			min	-29.627	3	0	1	-.613	3	0	1	0	3	0	1
225		18	max	805.756	1	0	1	.253	1	0	1	0	1	0	1
226			min	-29.579	3	0	1	-.613	3	0	1	0	3	0	1
227		19	max	805.821	1	0	1	.253	1	0	1	0	1	0	1
228			min	-29.53	3	0	1	-.613	3	0	1	0	3	0	1
229	M10	1	max	212.214	1	.655	4	-.004	15	0	1	0	1	0	1
230			min	-296.745	3	.154	15	-.114	1	0	3	0	3	0	1
231		2	max	212.34	1	.604	4	-.004	15	0	1	0	1	0	15
232			min	-296.651	3	.142	15	-.114	1	0	3	0	3	0	4
233		3	max	212.466	1	.553	4	-.004	15	0	1	0	1	0	15
234			min	-296.557	3	.13	15	-.114	1	0	3	0	3	0	4
235		4	max	212.592	1	.502	4	-.004	15	0	1	0	1	0	15
236			min	-296.462	3	.118	15	-.114	1	0	3	0	3	0	4
237		5	max	212.718	1	.451	4	-.004	15	0	1	0	1	0	15
238			min	-296.368	3	.106	15	-.114	1	0	3	0	3	0	4
239		6	max	212.844	1	.4	4	-.004	15	0	1	0	1	0	15
240			min	-296.273	3	.094	15	-.114	1	0	3	0	3	0	4
241		7	max	212.97	1	.349	4	-.004	15	0	1	0	1	0	15
242			min	-296.179	3	.082	15	-.114	1	0	3	0	3	0	4
243		8	max	213.096	1	.297	4	-.004	15	0	1	0	9	0	15
244			min	-296.085	3	.07	15	-.114	1	0	3	0	3	0	4
245		9	max	213.221	1	.246	4	-.004	15	0	1	0	9	0	15
246			min	-295.99	3	.058	15	-.114	1	0	3	0	3	0	4
247		10	max	213.347	1	.195	4	-.004	15	0	1	0	15	0	15
248			min	-295.896	3	.046	15	-.114	1	0	3	0	3	0	4
249		11	max	213.473	1	.144	4	-.004	15	0	1	0	15	0	15
250			min	-295.801	3	.034	15	-.114	1	0	3	0	3	0	4
251		12	max	213.599	1	.102	2	-.004	15	0	1	0	15	0	15
252			min	-295.707	3	.022	15	-.114	1	0	3	0	3	0	4
253		13	max	213.725	1	.062	2	-.004	15	0	1	0	15	0	15
254			min	-295.613	3	.005	12	-.114	1	0	3	0	3	0	4
255		14	max	213.851	1	.022	2	-.004	15	0	1	0	15	0	15
256			min	-295.518	3	-.023	3	-.114	1	0	3	0	3	0	4
257		15	max	213.977	1	-.014	15	-.004	15	0	1	0	15	0	15
258			min	-295.424	3	-.061	4	-.114	1	0	3	0	3	0	4
259		16	max	214.102	1	-.026	15	-.004	15	0	1	0	15	0	15
260			min	-295.329	3	-.112	4	-.114	1	0	3	0	3	0	4
261		17	max	214.228	1	-.038	15	-.004	15	0	1	0	15	0	15
262			min	-295.235	3	-.163	4	-.114	1	0	3	0	3	0	4
263		18	max	214.354	1	-.05	15	-.004	15	0	1	0	15	0	15
264			min	-295.141	3	-.214	4	-.114	1	0	3	0	3	0	4
265		19	max	214.48	1	-.062	15	-.004	15	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-295.046	3	-.265	4	-.114	1	0	3	0	1	0	4
267	M11	1	max	174.1	2	1.759	4	.21	1	0	3	0	3	0	4
268			min	-171.72	3	.414	15	-.046	3	0	10	0	1	0	15
269		2	max	174.031	2	1.582	4	.21	1	0	3	0	3	0	2
270			min	-171.772	3	.372	15	-.046	3	0	10	0	1	0	3
271		3	max	173.961	2	1.405	4	.21	1	0	3	0	3	0	2
272			min	-171.824	3	.33	15	-.046	3	0	10	0	1	0	3
273		4	max	173.892	2	1.228	4	.21	1	0	3	0	3	0	15
274			min	-171.876	3	.289	15	-.046	3	0	10	0	1	0	3
275		5	max	173.823	2	1.051	4	.21	1	0	3	0	3	0	15
276			min	-171.928	3	.247	15	-.046	3	0	10	0	1	0	4
277		6	max	173.753	2	.874	4	.21	1	0	3	0	3	0	15
278			min	-171.98	3	.206	15	-.046	3	0	10	0	1	0	4
279		7	max	173.684	2	.698	4	.21	1	0	3	0	3	0	15
280			min	-172.032	3	.164	15	-.046	3	0	10	0	1	0	4
281		8	max	173.615	2	.521	4	.21	1	0	3	0	3	0	15
282			min	-172.084	3	.123	15	-.046	3	0	10	0	1	-.001	4
283		9	max	173.545	2	.344	4	.21	1	0	3	0	3	0	15
284			min	-172.136	3	.081	15	-.046	3	0	10	0	1	-.001	4
285		10	max	173.476	2	.167	4	.21	1	0	3	0	3	0	15
286			min	-172.188	3	.039	15	-.046	3	0	10	0	1	-.001	4
287		11	max	173.407	2	.017	2	.21	1	0	3	0	3	0	15
288			min	-172.24	3	-.048	3	-.046	3	0	10	0	1	-.001	4
289		12	max	173.337	2	-.044	15	.21	1	0	3	0	3	0	15
290			min	-172.292	3	-.187	4	-.046	3	0	10	0	1	-.001	4
291		13	max	173.268	2	-.085	15	.21	1	0	3	0	3	0	15
292			min	-172.344	3	-.363	4	-.046	3	0	10	0	1	-.001	4
293		14	max	173.199	2	-.127	15	.21	1	0	3	0	3	0	15
294			min	-172.396	3	-.54	4	-.046	3	0	10	0	1	-.001	4
295		15	max	173.129	2	-.168	15	.21	1	0	3	0	3	0	15
296			min	-172.448	3	-.717	4	-.046	3	0	10	0	1	0	4
297		16	max	173.06	2	-.21	15	.21	1	0	3	0	3	0	15
298			min	-172.5	3	-.894	4	-.046	3	0	10	0	10	0	4
299		17	max	172.991	2	-.252	15	.21	1	0	3	0	3	0	15
300			min	-172.552	3	-1.071	4	-.046	3	0	10	0	10	0	4
301		18	max	172.921	2	-.293	15	.21	1	0	3	0	3	0	15
302			min	-172.604	3	-1.248	4	-.046	3	0	10	0	10	0	4
303		19	max	172.852	2	-.335	15	.21	1	0	3	0	3	0	1
304			min	-172.656	3	-1.424	4	-.046	3	0	10	0	10	0	1
305	M12	1	max	294.087	1	0	1	1.183	1	0	1	0	2	0	1
306			min	3.34	12	0	1	.027	10	0	1	0	3	0	1
307		2	max	294.152	1	0	1	1.183	1	0	1	0	1	0	1
308			min	3.372	12	0	1	.027	10	0	1	0	15	0	1
309		3	max	294.216	1	0	1	1.183	1	0	1	0	1	0	1
310			min	3.405	12	0	1	.027	10	0	1	0	15	0	1
311		4	max	294.281	1	0	1	1.183	1	0	1	0	1	0	1
312			min	3.437	12	0	1	.027	10	0	1	0	15	0	1
313		5	max	294.346	1	0	1	1.183	1	0	1	0	1	0	1
314			min	3.469	12	0	1	.027	10	0	1	0	10	0	1
315		6	max	294.411	1	0	1	1.183	1	0	1	0	1	0	1
316			min	3.502	12	0	1	.027	10	0	1	0	10	0	1
317		7	max	294.475	1	0	1	1.183	1	0	1	0	1	0	1
318			min	3.534	12	0	1	.027	10	0	1	0	10	0	1
319		8	max	294.54	1	0	1	1.183	1	0	1	0	1	0	1
320			min	3.566	12	0	1	.027	10	0	1	0	10	0	1
321		9	max	294.605	1	0	1	1.183	1	0	1	0	1	0	1
322			min	3.599	12	0	1	.027	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323		10	max	294.669	1	0	1	1.183	1	0	1	0	1	0	1
324			min	3.631	12	0	1	.027	10	0	1	0	10	0	1
325		11	max	294.734	1	0	1	1.183	1	0	1	.001	1	0	1
326			min	3.664	12	0	1	.027	10	0	1	0	10	0	1
327		12	max	294.799	1	0	1	1.183	1	0	1	.001	1	0	1
328			min	3.696	12	0	1	.027	10	0	1	0	10	0	1
329		13	max	294.863	1	0	1	1.183	1	0	1	.001	1	0	1
330			min	3.728	12	0	1	.027	10	0	1	0	10	0	1
331		14	max	294.928	1	0	1	1.183	1	0	1	.001	1	0	1
332			min	3.761	12	0	1	.027	10	0	1	0	10	0	1
333		15	max	294.993	1	0	1	1.183	1	0	1	.001	1	0	1
334			min	3.793	12	0	1	.027	10	0	1	0	10	0	1
335		16	max	295.058	1	0	1	1.183	1	0	1	.002	1	0	1
336			min	3.825	12	0	1	.027	10	0	1	0	10	0	1
337		17	max	295.122	1	0	1	1.183	1	0	1	.002	1	0	1
338			min	3.858	12	0	1	.027	10	0	1	0	10	0	1
339		18	max	295.187	1	0	1	1.183	1	0	1	.002	1	0	1
340			min	3.89	12	0	1	.027	10	0	1	0	10	0	1
341		19	max	295.252	1	0	1	1.183	1	0	1	.002	1	0	1
342			min	3.922	12	0	1	.027	10	0	1	0	10	0	1
343	M1	1	max	87.423	1	332.148	3	-.932	10	0	2	.049	1	0	2
344			min	3.586	15	-222.248	2	-25.129	1	0	3	.002	10	0	3
345		2	max	87.563	1	331.966	3	-.932	10	0	2	.044	1	.048	2
346			min	3.628	15	-222.49	2	-25.129	1	0	3	.002	10	-.072	3
347		3	max	86.573	3	4.696	9	-.926	10	0	12	.038	1	.096	2
348			min	-13.986	10	-23.775	2	-25.045	1	0	1	.001	10	-.143	3
349		4	max	86.677	3	4.494	9	-.926	10	0	12	.033	1	.101	2
350			min	-13.87	10	-24.016	2	-25.045	1	0	1	.001	10	-.14	3
351		5	max	86.782	3	4.293	9	-.926	10	0	12	.027	1	.106	2
352			min	-13.754	10	-24.258	2	-25.045	1	0	1	0	10	-.137	3
353		6	max	86.887	3	4.091	9	-.926	10	0	12	.022	1	.112	2
354			min	-13.637	10	-24.5	2	-25.045	1	0	1	0	10	-.135	3
355		7	max	86.992	3	3.89	9	-.926	10	0	12	.016	1	.117	2
356			min	-13.521	10	-24.742	2	-25.045	1	0	1	0	10	-.132	3
357		8	max	87.096	3	3.688	9	-.926	10	0	12	.011	1	.122	2
358			min	-13.405	10	-24.984	2	-25.045	1	0	1	0	10	-.129	3
359		9	max	87.201	3	3.486	9	-.926	10	0	12	.006	1	.128	2
360			min	-13.288	10	-25.226	2	-25.045	1	0	1	0	10	-.126	3
361		10	max	87.306	3	3.285	9	-.926	10	0	12	.002	3	.133	2
362			min	-13.172	10	-25.467	2	-25.045	1	0	1	0	10	-.123	3
363		11	max	87.41	3	3.083	9	-.926	10	0	12	0	3	.139	2
364			min	-13.056	10	-25.709	2	-25.045	1	0	1	-.005	1	-.12	3
365		12	max	87.515	3	2.882	9	-.926	10	0	12	0	12	.145	2
366			min	-12.939	10	-25.951	2	-25.045	1	0	1	-.011	1	-.117	3
367		13	max	87.62	3	2.68	9	-.926	10	0	12	0	10	.15	2
368			min	-12.823	10	-26.193	2	-25.045	1	0	1	-.016	1	-.114	3
369		14	max	87.725	3	2.479	9	-.926	10	0	12	0	10	.156	2
370			min	-12.707	10	-26.435	2	-25.045	1	0	1	-.022	1	-.11	3
371		15	max	87.829	3	2.277	9	-.926	10	0	12	-.001	10	.162	2
372			min	-12.59	10	-26.676	2	-25.045	1	0	1	-.027	1	-.107	3
373		16	max	87.501	2	117.665	2	-.933	10	0	1	-.001	10	.166	2
374			min	-5.744	3	-159.127	3	-25.217	1	0	12	-.033	1	-.103	3
375		17	max	87.64	2	117.424	2	-.933	10	0	1	-.001	10	.141	2
376			min	-5.64	3	-159.308	3	-25.217	1	0	12	-.038	1	-.068	3
377		18	max	-3.627	15	325.48	2	-.959	10	0	3	-.002	10	.071	2
378			min	-87.555	1	-156.813	3	-25.916	1	0	2	-.044	1	-.034	3
379		19	max	-3.585	15	325.238	2	-.959	10	0	3	-.002	10	0	2





Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	86.974	3	3.269	9	24.605	1	0	1	.01	3	.133	2
438		min	-12.79	10	-25.438	2	-2.195	3	0	10	0	1	-.123	3
439	11	max	87.079	3	3.067	9	24.605	1	0	1	.01	3	.139	2
440		min	-12.674	10	-25.68	2	-2.195	3	0	10	0	10	-.12	3
441	12	max	87.184	3	2.866	9	24.605	1	0	1	.011	1	.144	2
442		min	-12.557	10	-25.922	2	-2.195	3	0	10	0	10	-.117	3
443	13	max	87.288	3	2.664	9	24.605	1	0	1	.016	1	.15	2
444		min	-12.441	10	-26.164	2	-2.195	3	0	10	0	10	-.114	3
445	14	max	87.393	3	2.463	9	24.605	1	0	1	.021	1	.156	2
446		min	-12.325	10	-26.406	2	-2.195	3	0	10	0	10	-.111	3
447	15	max	87.498	3	2.261	9	24.605	1	0	1	.027	1	.162	2
448		min	-12.208	10	-26.648	2	-2.195	3	0	10	.001	10	-.107	3
449	16	max	87.706	2	117.31	2	24.789	1	0	15	.032	1	.166	2
450		min	-6.469	3	-159.644	3	-2.236	3	0	3	.001	10	-.103	3
451	17	max	87.846	2	117.068	2	24.789	1	0	15	.038	1	.141	2
452		min	-6.365	3	-159.825	3	-2.236	3	0	3	.001	10	-.068	3
453	18	max	-3.615	15	325.48	2	25.984	1	0	2	.043	1	.071	2
454		min	-87.346	1	-156.804	3	-1.774	3	0	3	.002	10	-.034	3
455	19	max	-3.573	15	325.238	2	25.984	1	0	2	.049	1	0	2
456		min	-87.207	1	-156.985	3	-1.774	3	0	3	.002	10	0	3
457	M13	1	max	72.867	3	222.149	2	-3.574	15	0	.049	1	0	2
458		min	.932	10	-332.115	3	-87.209	1	0	3	.002	10	0	3
459	2	max	72.867	3	157.936	2	-2.719	15	0	2	.013	3	.142	3
460		min	.932	10	-235.576	3	-65.977	1	0	3	-.002	10	-.095	2
461	3	max	72.867	3	93.724	2	-1.864	15	0	2	.009	3	.236	3
462		min	.932	10	-139.038	3	-44.744	1	0	3	-.017	1	-.158	2
463	4	max	72.867	3	29.512	2	-.873	10	0	2	.006	3	.281	3
464		min	.932	10	-42.5	3	-23.511	1	0	3	-.034	1	-.189	2
465	5	max	72.867	3	54.039	3	1.614	2	0	2	.003	3	.278	3
466		min	.932	10	-34.7	2	-4.518	3	0	3	-.041	1	-.187	2
467	6	max	72.867	3	150.577	3	18.954	1	0	2	.001	3	.227	3
468		min	.932	10	-98.912	2	-3.274	3	0	3	-.036	1	-.154	2
469	7	max	72.867	3	247.115	3	40.187	1	0	2	0	3	.127	3
470		min	.932	10	-163.124	2	-2.031	3	0	3	-.022	1	-.089	2
471	8	max	72.867	3	343.654	3	61.42	1	0	2	.005	2	.01	1
472		min	.932	10	-227.336	2	-.787	3	0	3	0	3	-.02	3
473	9	max	72.867	3	440.192	3	82.652	1	0	2	.04	1	.139	2
474		min	.932	10	-291.549	2	.457	3	0	3	0	3	-.216	3
475	10	max	72.867	3	536.73	3	103.885	1	0	2	.086	1	.301	2
476		min	.932	10	-355.761	2	1.421	12	0	3	-.01	3	-.46	3
477	11	max	25.182	1	291.549	2	.228	3	0	3	.039	1	.139	2
478		min	.932	10	-440.192	3	-82.444	1	0	2	-.01	3	-.216	3
479	12	max	25.182	1	227.336	2	1.472	3	0	3	.005	2	.01	1
480		min	.932	10	-343.654	3	-61.212	1	0	2	-.009	3	-.02	3
481	13	max	25.182	1	163.124	2	2.716	3	0	3	0	10	.127	3
482		min	.932	10	-247.115	3	-39.979	1	0	2	-.022	1	-.089	2
483	14	max	25.182	1	98.912	2	3.959	3	0	3	-.001	15	.227	3
484		min	.932	10	-150.577	3	-18.746	1	0	2	-.036	1	-.154	2
485	15	max	25.182	1	34.7	2	5.203	3	0	3	-.002	15	.278	3
486		min	.932	10	-54.039	3	-1.614	2	0	2	-.041	1	-.187	2
487	16	max	25.182	1	42.5	3	23.719	1	0	3	0	12	.281	3
488		min	.932	10	-29.512	2	.873	10	0	2	-.034	1	-.189	2
489	17	max	25.182	1	139.038	3	44.952	1	0	3	.002	3	.236	3
490		min	.932	10	-93.724	2	1.877	15	0	2	-.017	1	-.158	2
491	18	max	25.182	1	235.576	3	66.185	1	0	3	.011	1	.142	3
492		min	.932	10	-157.936	2	2.732	15	0	2	-.002	10	-.095	2
493	19	max	25.182	1	332.115	3	87.417	1	0	3	.049	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	.932	10	-222.149	2	3.586	15	0	2	.002	10	0	3
495		max	1.777	3	325.363	2	-3.573	15	0	3	.049	1	0	2
496		min	-25.93	1	-157.011	3	-87.213	1	0	2	.002	10	0	3
497		2 max	1.777	3	231.114	2	-2.719	15	0	3	.011	1	.067	3
498		min	-25.93	1	-111.964	3	-65.98	1	0	2	-.002	10	-.139	2
499		3 max	1.777	3	136.864	2	-1.864	15	0	3	0	3	.112	3
500		min	-25.93	1	-66.917	3	-44.748	1	0	2	-.017	1	-.231	2
501		4 max	1.777	3	42.615	2	-.886	10	0	3	-.001	15	.134	3
502		min	-25.93	1	-21.87	3	-23.515	1	0	2	-.034	1	-.276	2
503		5 max	1.777	3	23.176	3	1.589	2	0	3	-.002	15	.134	3
504		min	-25.93	1	-51.634	2	-3.024	3	0	2	-.041	1	-.274	2
505		6 max	1.777	3	68.223	3	18.95	1	0	3	-.001	15	.111	3
506		min	-25.93	1	-145.883	2	-1.78	3	0	2	-.036	1	-.224	2
507		7 max	1.777	3	113.27	3	40.183	1	0	3	0	10	.066	3
508		min	-25.93	1	-240.132	2	-.536	3	0	2	-.022	1	-.128	2
509		8 max	1.777	3	158.317	3	61.416	1	0	3	.005	2	.016	2
510		min	-25.93	1	-334.381	2	.658	12	0	2	-.006	3	-.002	3
511		9 max	1.777	3	203.364	3	82.648	1	0	3	.04	1	.207	2
512		min	-25.93	1	-428.63	2	1.487	12	0	2	-.005	3	-.093	3
513		10 max	-.959	10	-8.222	15	103.881	1	0	15	.086	1	.444	2
514		min	-25.93	1	-522.88	2	-4.224	3	0	2	-.004	3	-.206	3
515		11 max	-.959	10	428.63	2	-2.102	12	0	2	.039	1	.207	2
516		min	-25.864	1	-203.364	3	-82.44	1	0	3	0	3	-.093	3
517		12 max	-.959	10	334.381	2	-1.272	12	0	2	.005	2	.016	2
518		min	-25.864	1	-158.317	3	-61.207	1	0	3	-.001	3	-.002	3
519		13 max	-.959	10	240.132	2	-.443	12	0	2	0	10	.066	3
520		min	-25.864	1	-113.27	3	-39.975	1	0	3	-.022	1	-.128	2
521		14 max	-.959	10	145.883	2	.75	3	0	2	-.001	12	.111	3
522		min	-25.864	1	-68.223	3	-18.742	1	0	3	-.036	1	-.224	2
523		15 max	-.959	10	51.634	2	2.706	9	0	2	0	12	.134	3
524		min	-25.864	1	-23.176	3	-1.588	2	0	3	-.04	1	-.274	2
525		16 max	-.959	10	21.871	3	23.723	1	0	2	0	3	.134	3
526		min	-25.864	1	-42.615	2	.886	10	0	3	-.034	1	-.276	2
527		17 max	-.959	10	66.917	3	44.956	1	0	2	.002	3	.112	3
528		min	-25.864	1	-136.864	2	1.876	15	0	3	-.017	1	-.231	2
529		18 max	-.959	10	111.964	3	66.189	1	0	2	.011	1	.067	3
530		min	-25.864	1	-231.114	2	2.731	15	0	3	-.002	10	-.139	2
531		19 max	-.959	10	157.011	3	87.422	1	0	2	.049	1	0	2
532		min	-25.864	1	-325.363	2	3.585	15	0	3	.002	10	0	3
533	M15	1 max	0	1	.933	3	.108	3	0	1	0	1	0	1
534		min	-93.603	3	0	1	0	1	0	3	0	3	0	1
535		2 max	0	1	.829	3	.108	3	0	1	0	1	0	1
536		min	-93.674	3	0	1	0	1	0	3	0	3	0	3
537		3 max	0	1	.726	3	.108	3	0	1	0	1	0	1
538		min	-93.744	3	0	1	0	1	0	3	0	3	0	3
539		4 max	0	1	.622	3	.108	3	0	1	0	1	0	1
540		min	-93.815	3	0	1	0	1	0	3	0	3	0	3
541		5 max	0	1	.518	3	.108	3	0	1	0	1	0	1
542		min	-93.885	3	0	1	0	1	0	3	0	3	0	3
543		6 max	0	1	.415	3	.108	3	0	1	0	1	0	1
544		min	-93.956	3	0	1	0	1	0	3	0	3	-.001	3
545		7 max	0	1	.311	3	.108	3	0	1	0	3	0	1
546		min	-94.026	3	0	1	0	1	0	3	0	1	-.001	3
547		8 max	0	1	.207	3	.108	3	0	1	0	3	0	1
548		min	-94.097	3	0	1	0	1	0	3	0	1	-.001	3
549		9 max	0	1	.104	3	.108	3	0	1	0	3	0	1
550		min	-94.167	3	0	1	0	1	0	3	0	1	-.001	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.108	3	0	1	0	3	0	1
552		min	-94.238	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.108	3	0	1	0	3	0	1
554		min	-94.308	3	-.104	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.108	3	0	1	0	3	0	1
556		min	-94.379	3	-.207	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.108	3	0	1	0	3	0	1
558		min	-94.449	3	-.311	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.108	3	0	1	0	3	0	1
560		min	-94.52	3	-.415	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.108	3	0	1	0	3	0	1
562		min	-94.59	3	-.518	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.108	3	0	1	0	3	0	1
564		min	-94.661	3	-.622	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.108	3	0	1	0	3	0	1
566		min	-94.731	3	-.726	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.108	3	0	1	0	3	0	1
568		min	-94.802	3	-.829	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.108	3	0	1	0	3	0	1
570		min	-94.872	3	-.933	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.597	4	.034	1	0	3	0	3	0	1
572		min	-93.563	3	0	2	-.044	3	0	1	0	1	0	1
573	2	max	0	2	1.419	4	.034	1	0	3	0	3	0	2
574		min	-93.493	3	0	2	-.044	3	0	1	0	1	0	4
575	3	max	0	2	1.242	4	.034	1	0	3	0	3	0	2
576		min	-93.422	3	0	2	-.044	3	0	1	0	1	0	4
577	4	max	0	2	1.064	4	.034	1	0	3	0	3	0	2
578		min	-93.352	3	0	2	-.044	3	0	1	0	1	-.001	4
579	5	max	0	2	.887	4	.034	1	0	3	0	3	0	2
580		min	-93.281	3	0	2	-.044	3	0	1	0	1	-.002	4
581	6	max	0	2	.71	4	.034	1	0	3	0	3	0	2
582		min	-93.211	3	0	2	-.044	3	0	1	0	1	-.002	4
583	7	max	0	2	.532	4	.034	1	0	3	0	3	0	2
584		min	-93.14	3	0	2	-.044	3	0	1	0	1	-.002	4
585	8	max	0	2	.355	4	.034	1	0	3	0	3	0	2
586		min	-93.07	3	0	2	-.044	3	0	1	0	1	-.002	4
587	9	max	0	2	.177	4	.034	1	0	3	0	3	0	2
588		min	-92.999	3	0	2	-.044	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.034	1	0	3	0	3	0	2
590		min	-92.929	3	0	1	-.044	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.034	1	0	3	0	3	0	2
592		min	-92.858	3	-.177	4	-.044	3	0	1	0	1	-.002	4
593	12	max	0	2	0	2	.034	1	0	3	0	3	0	2
594		min	-92.788	3	-.355	4	-.044	3	0	1	0	1	-.002	4
595	13	max	.095	13	0	2	.034	1	0	3	0	1	0	2
596		min	-92.717	3	-.532	4	-.044	3	0	1	0	4	-.002	4
597	14	max	.192	13	0	2	.034	1	0	3	0	1	0	2
598		min	-92.647	3	-.71	4	-.044	3	0	1	0	3	-.002	4
599	15	max	.289	13	0	2	.034	1	0	3	0	1	0	2
600		min	-92.576	3	-.887	4	-.044	3	0	1	0	3	-.002	4
601	16	max	.386	13	0	2	.034	1	0	3	0	1	0	2
602		min	-92.506	3	-1.064	4	-.044	3	0	1	0	3	-.001	4
603	17	max	.483	13	0	2	.034	1	0	3	0	1	0	2
604		min	-92.435	3	-1.242	4	-.044	3	0	1	0	3	0	4
605	18	max	.58	13	0	2	.034	1	0	3	0	1	0	2
606		min	-92.365	3	-1.419	4	-.044	3	0	1	0	3	0	4
607	19	max	.677	13	0	2	.034	1	0	3	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-92.294	3	-1.597	4	-.044	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.009	2	.004	1	-1.44e-5	10	NC	3	NC	2
2			min	-.003	3	-.009	3	-.002	3	-3.99e-4	1	4338.682	2	8860.739	1
3		2	max	.002	1	.008	2	.004	1	-1.372e-5	10	NC	3	NC	2
4			min	-.003	3	-.009	3	-.002	3	-3.814e-4	1	4739.406	2	9536.247	1
5		3	max	.002	1	.008	2	.004	1	-1.304e-5	10	NC	1	NC	1
6			min	-.003	3	-.008	3	-.002	3	-3.637e-4	1	5216.833	2	NC	1
7		4	max	.002	1	.007	2	.003	1	-1.236e-5	10	NC	1	NC	1
8			min	-.003	3	-.008	3	-.001	3	-3.461e-4	1	5789.68	2	NC	1
9		5	max	.002	1	.006	2	.003	1	-1.168e-5	10	NC	1	NC	1
10			min	-.003	3	-.008	3	-.001	3	-3.285e-4	1	6482.961	2	NC	1
11		6	max	.001	1	.005	2	.003	1	-1.1e-5	10	NC	1	NC	1
12			min	-.002	3	-.007	3	-.001	3	-3.109e-4	1	7330.659	2	NC	1
13		7	max	.001	1	.005	2	.003	1	-1.032e-5	10	NC	1	NC	1
14			min	-.002	3	-.007	3	0	3	-2.933e-4	1	8379.835	2	NC	1
15		8	max	.001	1	.004	2	.002	1	-9.645e-6	10	NC	1	NC	1
16			min	-.002	3	-.006	3	0	3	-2.757e-4	1	9697.136	2	NC	1
17		9	max	.001	1	.003	2	.002	1	-8.966e-6	10	NC	1	NC	1
18			min	-.002	3	-.006	3	0	3	-2.581e-4	1	NC	1	NC	1
19		10	max	.001	1	.003	2	.002	1	-8.286e-6	10	NC	1	NC	1
20			min	-.002	3	-.006	3	0	3	-2.404e-4	1	NC	1	NC	1
21		11	max	0	1	.002	2	.001	1	-7.607e-6	10	NC	1	NC	1
22			min	-.002	3	-.005	3	0	3	-2.228e-4	1	NC	1	NC	1
23		12	max	0	1	.002	2	.001	1	-6.928e-6	10	NC	1	NC	1
24			min	-.001	3	-.004	3	0	3	-2.052e-4	1	NC	1	NC	1
25		13	max	0	1	.001	2	0	1	-6.248e-6	10	NC	1	NC	1
26			min	-.001	3	-.004	3	0	3	-1.876e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	0	1	-5.569e-6	10	NC	1	NC	1
28			min	0	3	-.003	3	0	3	-1.7e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	0	1	-4.89e-6	10	NC	1	NC	1
30			min	0	3	-.003	3	0	3	-1.524e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	0	1	-4.21e-6	10	NC	1	NC	1
32			min	0	3	-.002	3	0	3	-1.348e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	-3.531e-6	10	NC	1	NC	1
34			min	0	3	-.001	3	0	3	-1.172e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	-2.851e-6	10	NC	1	NC	1
36			min	0	3	0	3	0	3	-9.954e-5	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	-2.172e-6	10	NC	1	NC	1
38			min	0	1	0	1	0	1	-8.193e-5	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	3.869e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	1.03e-6	10	NC	1	NC	1
41		2	max	0	3	0	2	0	10	4.915e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	1.525e-6	10	NC	1	NC	1
43		3	max	0	3	0	2	0	10	5.961e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	2.02e-6	10	NC	1	NC	1
45		4	max	0	3	0	2	0	3	7.006e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	2.515e-6	10	NC	1	NC	1
47		5	max	0	3	0	2	0	3	8.052e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	3.01e-6	10	NC	1	NC	1
49		6	max	0	3	0	2	0	3	9.097e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	3.505e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	1.014e-4	1	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	9	4.e-6	10	NC	1	NC	1
53	8	max	0	3	0	2	0	3	1.119e-4	1	NC	1	NC	1
54		min	0	2	-.006	3	0	9	4.496e-6	10	NC	1	NC	1
55	9	max	0	3	.001	2	0	3	1.223e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	9	4.991e-6	10	NC	1	NC	1
57	10	max	0	3	.002	2	0	1	1.328e-4	1	NC	1	NC	1
58		min	0	2	-.007	3	0	15	5.486e-6	10	NC	1	NC	1
59	11	max	.001	3	.002	2	0	1	1.433e-4	1	NC	1	NC	1
60		min	-.001	2	-.007	3	0	15	5.981e-6	10	NC	1	NC	1
61	12	max	.001	3	.003	2	0	1	1.537e-4	1	NC	1	NC	1
62		min	-.001	2	-.007	3	0	15	6.456e-6	15	NC	1	NC	1
63	13	max	.001	3	.003	2	.001	1	1.642e-4	1	NC	1	NC	1
64		min	-.001	2	-.008	3	0	15	6.884e-6	15	NC	1	NC	1
65	14	max	.001	3	.004	2	.001	1	1.746e-4	1	NC	1	NC	1
66		min	-.001	2	-.008	3	0	15	7.311e-6	15	NC	1	NC	1
67	15	max	.002	3	.005	2	.002	1	1.851e-4	1	NC	1	NC	1
68		min	-.002	2	-.008	3	0	15	7.739e-6	15	9475.919	2	NC	1
69	16	max	.002	3	.006	2	.002	1	1.955e-4	1	NC	1	NC	1
70		min	-.002	2	-.008	3	0	15	8.166e-6	15	7984.906	2	NC	1
71	17	max	.002	3	.007	2	.002	1	2.06e-4	1	NC	1	NC	1
72		min	-.002	2	-.008	3	0	15	8.594e-6	15	6841.071	2	NC	1
73	18	max	.002	3	.008	2	.003	1	2.164e-4	1	NC	1	NC	1
74		min	-.002	2	-.008	3	0	15	9.021e-6	15	5952.335	2	NC	1
75	19	max	.002	3	.009	2	.003	1	2.269e-4	1	NC	3	NC	1
76		min	-.002	2	-.008	3	0	15	9.448e-6	15	5254.883	2	NC	1
77	M4	1	max	.001	1	.01	2	15	-1.089e-5	10	NC	1	NC	2
78		min	0	12	-.009	3	-.002	1	-3.116e-4	1	NC	1	9003.771	1
79	2	max	.001	1	.01	2	0	15	-1.089e-5	10	NC	1	NC	2
80		min	0	12	-.009	3	-.002	1	-3.116e-4	1	NC	1	9822.156	1
81	3	max	.001	1	.009	2	0	15	-1.089e-5	10	NC	1	NC	1
82		min	0	12	-.008	3	-.002	1	-3.116e-4	1	NC	1	NC	1
83	4	max	.001	1	.009	2	0	15	-1.089e-5	10	NC	1	NC	1
84		min	0	12	-.008	3	-.002	1	-3.116e-4	1	NC	1	NC	1
85	5	max	.001	1	.008	2	0	15	-1.089e-5	10	NC	1	NC	1
86		min	0	12	-.007	3	-.001	1	-3.116e-4	1	NC	1	NC	1
87	6	max	.001	1	.008	2	0	15	-1.089e-5	10	NC	1	NC	1
88		min	0	12	-.007	3	-.001	1	-3.116e-4	1	NC	1	NC	1
89	7	max	0	1	.007	2	0	15	-1.089e-5	10	NC	1	NC	1
90		min	0	12	-.006	3	-.001	1	-3.116e-4	1	NC	1	NC	1
91	8	max	0	1	.006	2	0	15	-1.089e-5	10	NC	1	NC	1
92		min	0	12	-.006	3	0	1	-3.116e-4	1	NC	1	NC	1
93	9	max	0	1	.006	2	0	15	-1.089e-5	10	NC	1	NC	1
94		min	0	12	-.005	3	0	1	-3.116e-4	1	NC	1	NC	1
95	10	max	0	1	.005	2	0	15	-1.089e-5	10	NC	1	NC	1
96		min	0	12	-.005	3	0	1	-3.116e-4	1	NC	1	NC	1
97	11	max	0	1	.005	2	0	15	-1.089e-5	10	NC	1	NC	1
98		min	0	12	-.004	3	0	1	-3.116e-4	1	NC	1	NC	1
99	12	max	0	1	.004	2	0	15	-1.089e-5	10	NC	1	NC	1
100		min	0	12	-.004	3	0	1	-3.116e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-1.089e-5	10	NC	1	NC	1
102		min	0	12	-.003	3	0	1	-3.116e-4	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-1.089e-5	10	NC	1	NC	1
104		min	0	12	-.003	3	0	1	-3.116e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-1.089e-5	10	NC	1	NC	1
106		min	0	12	-.002	3	0	1	-3.116e-4	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-1.089e-5	10	NC	1	NC	1
108		min	0	12	-.002	3	0	1	-3.116e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-1.089e-5	10	NC	1	NC	1
110			min	0	12	-.001	3	0	1	-3.116e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.089e-5	10	NC	1	NC	1
112			min	0	12	0	3	0	1	-3.116e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.089e-5	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-3.116e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.03	2	.001	1	4.384e-4	3	NC	3	NC	1
116			min	-.011	3	-.028	3	-.006	3	-8.019e-8	2	1309.377	2	6938.294	3
117		2	max	.006	1	.028	2	.001	1	4.249e-4	3	NC	3	NC	1
118			min	-.01	3	-.026	3	-.005	3	-3.452e-7	11	1401.762	2	7365.676	3
119		3	max	.006	1	.026	2	.001	1	4.113e-4	3	NC	3	NC	1
120			min	-.009	3	-.025	3	-.005	3	-2.194e-6	1	1507.717	2	7873.49	3
121		4	max	.005	1	.024	2	.001	1	3.978e-4	3	NC	3	NC	1
122			min	-.009	3	-.024	3	-.005	3	-4.442e-6	1	1629.965	2	8478.875	3
123		5	max	.005	1	.022	2	.001	1	3.842e-4	3	NC	3	NC	1
124			min	-.008	3	-.022	3	-.004	3	-6.69e-6	1	1772.015	2	9204.58	3
125		6	max	.005	1	.02	2	0	1	3.707e-4	3	NC	3	NC	1
126			min	-.008	3	-.021	3	-.004	3	-8.937e-6	1	1938.462	2	NC	1
127		7	max	.004	1	.018	2	0	1	3.571e-4	3	NC	3	NC	1
128			min	-.007	3	-.019	3	-.004	3	-1.119e-5	1	2135.438	2	NC	1
129		8	max	.004	1	.017	2	0	1	3.436e-4	3	NC	3	NC	1
130			min	-.006	3	-.018	3	-.003	3	-1.343e-5	1	2371.312	2	NC	1
131		9	max	.004	1	.015	2	0	1	3.3e-4	3	NC	3	NC	1
132			min	-.006	3	-.016	3	-.003	3	-1.568e-5	1	2657.799	2	NC	1
133		10	max	.003	1	.013	2	0	1	3.165e-4	3	NC	3	NC	1
134			min	-.005	3	-.014	3	-.002	3	-1.793e-5	1	3011.823	2	NC	1
135		11	max	.003	1	.011	2	0	1	3.029e-4	3	NC	3	NC	1
136			min	-.005	3	-.013	3	-.002	3	-2.018e-5	1	3458.764	2	NC	1
137		12	max	.003	1	.01	2	0	1	2.894e-4	3	NC	3	NC	1
138			min	-.004	3	-.011	3	-.002	3	-2.242e-5	1	4038.497	2	NC	1
139		13	max	.002	1	.008	2	0	1	2.758e-4	3	NC	3	NC	1
140			min	-.004	3	-.01	3	-.001	3	-2.467e-5	1	4817.462	2	NC	1
141		14	max	.002	1	.007	2	0	1	2.623e-4	3	NC	3	NC	1
142			min	-.003	3	-.008	3	-.001	3	-2.692e-5	1	5915.221	2	NC	1
143		15	max	.001	1	.005	2	0	1	2.487e-4	3	NC	1	NC	1
144			min	-.002	3	-.007	3	0	3	-2.917e-5	1	7570.842	2	NC	1
145		16	max	.001	1	.004	2	0	1	2.352e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-3.141e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	2.216e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-3.366e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.081e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-3.591e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.945e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.816e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.79e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.128e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.582e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.835e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.374e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.542e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	1.166e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.248e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	9.578e-6	1	NC	1	NC	1
162			min	-.001	2	-.008	3	0	1	0	2	8948.368	2	NC	1
163		6	max	.002	3	.006	2	.002	3	2.338e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	0	2	7168.021	2	NC	1
165		7	max	.002	3	.008	2	.002	3	4.631e-5	3	NC	1	NC	1



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Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.011	3	0	1	-1.018e-7	13	5949.835	2	NC	1
167		8	max	.002	3	.009	2	.002	3	6.924e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-1.245e-6	9	5056.41	2	NC	1
169		9	max	.003	3	.011	2	.003	3	9.217e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	1	-3.153e-6	9	4369.467	2	NC	1
171		10	max	.003	3	.012	2	.003	3	1.151e-4	3	NC	3	NC	1
172			min	-.003	2	-.016	3	0	1	-5.062e-6	9	3823.436	2	NC	1
173		11	max	.003	3	.014	2	.003	3	1.38e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	1	-6.97e-6	9	3378.917	2	NC	1
175		12	max	.003	3	.015	2	.003	3	1.61e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	1	-8.878e-6	9	3010.672	2	NC	1
177		13	max	.004	3	.017	2	.003	3	1.839e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	0	1	-1.079e-5	9	2701.65	2	NC	1
179		14	max	.004	3	.019	2	.003	3	2.068e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	1	-1.269e-5	9	2439.813	2	NC	1
181		15	max	.004	3	.021	2	.003	3	2.298e-4	3	NC	3	NC	1
182			min	-.005	2	-.021	3	0	1	-1.46e-5	9	2216.339	2	NC	1
183		16	max	.005	3	.023	2	.003	3	2.527e-4	3	NC	3	NC	1
184			min	-.006	2	-.022	3	0	1	-1.651e-5	9	2024.567	2	NC	1
185		17	max	.005	3	.025	2	.003	3	2.756e-4	3	NC	3	NC	1
186			min	-.006	2	-.023	3	0	1	-1.842e-5	9	1859.339	2	NC	1
187		18	max	.005	3	.027	2	.003	3	2.986e-4	3	NC	3	NC	1
188			min	-.006	2	-.024	3	0	1	-2.033e-5	9	1716.588	2	NC	1
189		19	max	.006	3	.029	2	.003	3	3.215e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	1	-2.224e-5	9	1593.061	2	NC	1
191	M8	1	max	.004	1	.034	2	0	1	-1.037e-7	10	NC	1	NC	1
192			min	0	3	-.028	3	-.002	3	-2.414e-4	3	NC	1	9975.813	3
193		2	max	.004	1	.032	2	0	1	-1.037e-7	10	NC	1	NC	1
194			min	0	3	-.026	3	-.002	3	-2.414e-4	3	NC	1	NC	1
195		3	max	.003	1	.031	2	0	1	-1.037e-7	10	NC	1	NC	1
196			min	0	3	-.025	3	-.002	3	-2.414e-4	3	NC	1	NC	1
197		4	max	.003	1	.029	2	0	1	-1.037e-7	10	NC	1	NC	1
198			min	0	3	-.023	3	-.001	3	-2.414e-4	3	NC	1	NC	1
199		5	max	.003	1	.027	2	0	1	-1.037e-7	10	NC	1	NC	1
200			min	0	3	-.022	3	-.001	3	-2.414e-4	3	NC	1	NC	1
201		6	max	.003	1	.025	2	0	1	-1.037e-7	10	NC	1	NC	1
202			min	0	3	-.02	3	-.001	3	-2.414e-4	3	NC	1	NC	1
203		7	max	.003	1	.023	2	0	1	-1.037e-7	10	NC	1	NC	1
204			min	0	3	-.019	3	-.001	3	-2.414e-4	3	NC	1	NC	1
205		8	max	.002	1	.021	2	0	1	-1.037e-7	10	NC	1	NC	1
206			min	0	3	-.017	3	0	3	-2.414e-4	3	NC	1	NC	1
207		9	max	.002	1	.019	2	0	1	-1.037e-7	10	NC	1	NC	1
208			min	0	3	-.015	3	0	3	-2.414e-4	3	NC	1	NC	1
209		10	max	.002	1	.017	2	0	1	-1.037e-7	10	NC	1	NC	1
210			min	0	3	-.014	3	0	3	-2.414e-4	3	NC	1	NC	1
211		11	max	.002	1	.015	2	0	1	-1.037e-7	10	NC	1	NC	1
212			min	0	3	-.012	3	0	3	-2.414e-4	3	NC	1	NC	1
213		12	max	.001	1	.013	2	0	1	-1.037e-7	10	NC	1	NC	1
214			min	0	3	-.011	3	0	3	-2.414e-4	3	NC	1	NC	1
215		13	max	.001	1	.011	2	0	1	-1.037e-7	10	NC	1	NC	1
216			min	0	3	-.009	3	0	3	-2.414e-4	3	NC	1	NC	1
217		14	max	.001	1	.01	2	0	1	-1.037e-7	10	NC	1	NC	1
218			min	0	3	-.008	3	0	3	-2.414e-4	3	NC	1	NC	1
219		15	max	0	1	.008	2	0	1	-1.037e-7	10	NC	1	NC	1
220			min	0	3	-.006	3	0	3	-2.414e-4	3	NC	1	NC	1
221		16	max	0	1	.006	2	0	1	-1.037e-7	10	NC	1	NC	1
222			min	0	3	-.005	3	0	3	-2.414e-4	3	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	1	-1.037e-7	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.414e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.037e-7	10	NC	1	NC	1
226			min	0	3	-.002	3	0	3	-2.414e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.037e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.414e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.009	2	0	3	3.984e-4	1	NC	3	NC	1
230			min	-.003	3	-.009	3	-.002	1	-5.114e-4	3	4343.431	2	NC	1
231		2	max	.002	1	.008	2	0	3	3.783e-4	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.001	1	-4.94e-4	3	4744.745	2	NC	1
233		3	max	.002	1	.008	2	0	3	3.582e-4	1	NC	1	NC	1
234			min	-.003	3	-.008	3	-.001	1	-4.766e-4	3	5222.905	2	NC	1
235		4	max	.002	1	.007	2	0	3	3.382e-4	1	NC	1	NC	1
236			min	-.002	3	-.008	3	-.001	1	-4.592e-4	3	5796.672	2	NC	1
237		5	max	.002	1	.006	2	0	3	3.181e-4	1	NC	1	NC	1
238			min	-.002	3	-.008	3	-.001	1	-4.418e-4	3	6491.118	2	NC	1
239		6	max	.001	1	.005	2	0	3	2.981e-4	1	NC	1	NC	1
240			min	-.002	3	-.007	3	-.001	1	-4.244e-4	3	7340.31	2	NC	1
241		7	max	.001	1	.005	2	0	3	2.78e-4	1	NC	1	NC	1
242			min	-.002	3	-.007	3	-.001	1	-4.07e-4	3	8391.434	2	NC	1
243		8	max	.001	1	.004	2	0	3	2.58e-4	1	NC	1	NC	1
244			min	-.002	3	-.006	3	0	1	-3.896e-4	3	9711.318	2	NC	1
245		9	max	.001	1	.003	2	0	3	2.379e-4	1	NC	1	NC	1
246			min	-.002	3	-.006	3	0	1	-3.722e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	2.178e-4	1	NC	1	NC	1
248			min	-.001	3	-.006	3	0	1	-3.548e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	1.978e-4	1	NC	1	NC	1
250			min	-.001	3	-.005	3	0	1	-3.375e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	1.777e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-3.201e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	1.577e-4	1	NC	1	NC	1
254			min	0	3	-.004	3	0	1	-3.027e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.376e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-2.853e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	1.175e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.679e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	9.749e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.505e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	7.743e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.331e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	5.737e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.157e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.732e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.983e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	9.359e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.788e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.095e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-3.252e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	4.831e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-4.715e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	2.567e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-6.178e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	3.027e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-7.641e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-3.459e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-9.105e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-3.945e-6	10	NC	1	NC	1





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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.357e-4	3	NC	1	NC	1
338			min	0	12	-.001	3	0	10	1.068e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.357e-4	3	NC	1	NC	1
340			min	0	12	0	3	0	10	1.068e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.357e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	1.068e-5	10	NC	1	NC	1
343	M1	1	max	.008	3	.024	3	.003	3	7.052e-3	2	NC	1	NC	1
344			min	-.008	2	-.02	2	-.001	1	-1.015e-2	3	NC	1	NC	1
345		2	max	.008	3	.014	3	.003	3	3.468e-3	2	NC	4	NC	1
346			min	-.008	2	-.012	2	-.003	1	-5.008e-3	3	4865.011	3	NC	1
347		3	max	.008	3	.005	3	.002	3	3.855e-5	3	NC	4	NC	1
348			min	-.008	2	-.004	2	-.004	1	-2.243e-4	1	2520.273	3	NC	1
349		4	max	.008	3	.003	2	.002	3	4.012e-5	3	NC	4	NC	1
350			min	-.008	2	-.003	3	-.005	1	-1.905e-4	1	1796.072	3	NC	1
351		5	max	.008	3	.01	2	.001	3	4.168e-5	3	NC	4	NC	1
352			min	-.008	2	-.009	3	-.005	1	-1.566e-4	1	1450.807	3	NC	1
353		6	max	.008	3	.015	2	0	3	4.325e-5	3	NC	4	NC	1
354			min	-.008	2	-.014	3	-.005	1	-1.228e-4	1	1257.687	2	NC	1
355		7	max	.008	3	.019	2	0	3	4.481e-5	3	NC	4	NC	1
356			min	-.008	2	-.018	3	-.004	1	-8.9e-5	1	1119.268	2	NC	1
357		8	max	.008	3	.022	2	0	3	4.637e-5	3	NC	4	NC	1
358			min	-.008	2	-.021	3	-.003	1	-5.518e-5	1	1032.181	2	NC	1
359		9	max	.008	3	.024	2	0	3	4.794e-5	3	NC	4	NC	1
360			min	-.008	2	-.022	3	-.002	1	-2.535e-5	9	980.204	2	NC	1
361		10	max	.008	3	.025	2	0	3	4.95e-5	3	NC	4	NC	1
362			min	-.008	2	-.023	3	-.001	1	-1.203e-6	9	955.571	2	NC	1
363		11	max	.008	3	.025	2	0	3	5.107e-5	3	NC	4	NC	1
364			min	-.008	2	-.022	3	0	9	1.618e-6	15	955.437	2	NC	1
365		12	max	.008	3	.023	2	.001	1	8.012e-5	1	NC	4	NC	1
366			min	-.008	2	-.02	3	0	15	3.096e-6	15	980.827	2	NC	1
367		13	max	.008	3	.02	2	.002	1	1.139e-4	1	NC	4	NC	1
368			min	-.008	2	-.017	3	0	15	4.575e-6	15	1037.222	2	NC	1
369		14	max	.008	3	.016	2	.003	1	1.478e-4	1	NC	4	NC	1
370			min	-.008	2	-.013	3	0	15	6.053e-6	15	1137.356	2	NC	1
371		15	max	.008	3	.01	2	.003	1	1.816e-4	1	NC	4	NC	1
372			min	-.008	2	-.009	3	0	15	7.532e-6	15	1309.271	2	NC	1
373		16	max	.008	3	.003	2	.003	1	2.055e-4	1	NC	4	NC	1
374			min	-.008	2	-.003	3	0	15	8.57e-6	15	1621.765	2	NC	1
375		17	max	.008	3	.004	3	.002	1	5.733e-5	3	NC	4	NC	1
376			min	-.008	2	-.006	2	0	15	-2.001e-5	9	2296.587	2	NC	1
377		18	max	.008	3	.012	3	0	1	5.053e-3	2	NC	4	NC	1
378			min	-.008	2	-.016	2	0	15	-2.56e-3	3	4450.423	2	NC	1
379		19	max	.008	3	.02	3	0	3	1.018e-2	2	NC	1	NC	1
380			min	-.008	2	-.027	2	0	1	-5.227e-3	3	NC	1	NC	1
381	M5	1	max	.025	3	.076	3	.003	3	5.891e-6	3	NC	1	NC	1
382			min	-.028	2	-.066	2	-.002	1	0	15	NC	1	NC	1
383		2	max	.025	3	.045	3	.005	3	1.226e-4	3	NC	4	NC	1
384			min	-.028	2	-.038	2	-.001	1	-2.468e-5	1	1533.258	3	NC	1
385		3	max	.025	3	.016	3	.006	3	2.371e-4	3	NC	5	NC	1
386			min	-.028	2	-.012	2	-.001	1	-4.895e-5	1	794.647	3	NC	1
387		4	max	.025	3	.011	2	.007	3	2.292e-4	3	NC	5	NC	1
388			min	-.028	2	-.009	3	-.001	1	-4.632e-5	1	566.919	3	NC	1
389		5	max	.025	3	.031	2	.007	3	2.214e-4	3	NC	5	NC	1
390			min	-.028	2	-.029	3	-.001	1	-4.39e-5	9	458.225	2	NC	1
391		6	max	.025	3	.048	2	.008	3	2.135e-4	3	NC	5	NC	1
392			min	-.028	2	-.045	3	-.001	1	-4.169e-5	9	388.724	2	9306.49	3
393		7	max	.025	3	.062	2	.008	3	2.057e-4	3	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.008	3	.004	3	0	10	6.967e-5	3	NC	4	NC	1
452			min	-.008	2	-.006	2	-.004	1	-1.193e-4	1	2297.31	2	NC	1
453		18	max	.008	3	.012	3	0	10	2.625e-3	3	NC	4	NC	1
454			min	-.008	2	-.016	2	-.003	1	-5.053e-3	2	4451.779	2	NC	1
455		19	max	.008	3	.02	3	0	3	5.225e-3	3	NC	1	NC	1
456			min	-.008	2	-.027	2	0	1	-1.018e-2	2	NC	1	NC	1
457	M13	1	max	.002	1	.024	3	.008	3	3.72e-3	3	NC	1	NC	1
458			min	-.003	3	-.02	2	-.008	2	-3.261e-3	2	NC	1	NC	1
459		2	max	.002	1	.083	3	.006	3	4.625e-3	3	NC	4	NC	1
460			min	-.003	3	-.061	2	-.006	2	-4.068e-3	2	1832.453	3	NC	1
461		3	max	.002	1	.132	3	.013	1	5.531e-3	3	NC	5	NC	2
462			min	-.003	3	-.096	2	-.005	10	-4.875e-3	2	998.978	3	6132.838	1
463		4	max	.002	1	.165	3	.021	1	6.436e-3	3	NC	5	NC	2
464			min	-.003	3	-.119	2	-.005	10	-5.681e-3	2	766.571	3	4342.193	1
465		5	max	.002	1	.178	3	.023	1	7.341e-3	3	NC	5	NC	2
466			min	-.003	3	-.13	2	-.006	10	-6.488e-3	2	701.055	3	3968.445	1
467		6	max	.002	1	.172	3	.019	1	8.246e-3	3	NC	5	NC	2
468			min	-.003	3	-.127	2	-.008	10	-7.294e-3	2	730.649	3	4560.177	1
469		7	max	.002	1	.15	3	.016	3	9.151e-3	3	NC	5	NC	2
470			min	-.003	3	-.113	2	-.013	2	-8.101e-3	2	858.571	3	7250.734	1
471		8	max	.002	1	.119	3	.019	3	1.006e-2	3	NC	4	NC	1
472			min	-.003	3	-.094	2	-.02	2	-8.908e-3	2	1136.552	3	9495.324	2
473		9	max	.002	1	.09	3	.022	3	1.096e-2	3	NC	4	NC	1
474			min	-.003	3	-.075	2	-.025	2	-9.714e-3	2	1637.009	3	6331.985	2
475		10	max	.002	1	.076	3	.025	3	1.187e-2	3	NC	4	NC	4
476			min	-.003	3	-.066	2	-.028	2	-1.052e-2	2	2056.49	3	5514.799	2
477		11	max	.001	1	.09	3	.027	3	1.096e-2	3	NC	4	NC	1
478			min	-.003	3	-.075	2	-.025	2	-9.714e-3	2	1637.007	3	5762.991	3
479		12	max	.001	1	.119	3	.028	3	1.006e-2	3	NC	4	NC	1
480			min	-.003	3	-.094	2	-.02	2	-8.908e-3	2	1136.551	3	5586.61	3
481		13	max	.001	1	.15	3	.027	3	9.157e-3	3	NC	5	NC	2
482			min	-.003	3	-.113	2	-.013	2	-8.101e-3	2	858.57	3	5823.58	3
483		14	max	.001	1	.172	3	.025	3	8.254e-3	3	NC	5	NC	2
484			min	-.003	3	-.127	2	-.008	10	-7.294e-3	2	730.649	3	4556.447	1
485		15	max	.001	1	.178	3	.023	1	7.35e-3	3	NC	5	NC	2
486			min	-.003	3	-.13	2	-.007	10	-6.488e-3	2	701.055	3	3973.11	1
487		16	max	.001	1	.165	3	.02	1	6.447e-3	3	NC	5	NC	2
488			min	-.003	3	-.119	2	-.005	10	-5.681e-3	2	766.571	3	4355.32	1
489		17	max	.001	1	.132	3	.014	3	5.544e-3	3	NC	5	NC	2
490			min	-.003	3	-.096	2	-.005	10	-4.875e-3	2	998.977	3	6165.837	1
491		18	max	.001	1	.083	3	.011	3	4.641e-3	3	NC	4	NC	1
492			min	-.003	3	-.061	2	-.006	2	-4.068e-3	2	1832.451	3	NC	1
493		19	max	.001	1	.024	3	.008	3	3.738e-3	3	NC	1	NC	1
494			min	-.003	3	-.02	2	-.008	2	-3.262e-3	2	NC	1	NC	1
495	M16	1	max	0	1	.02	3	.008	3	4.131e-3	2	NC	1	NC	1
496			min	0	3	-.027	2	-.008	2	-3.025e-3	3	NC	1	NC	1
497		2	max	0	1	.05	3	.011	3	5.159e-3	2	NC	4	NC	1
498			min	0	3	-.086	2	-.006	2	-3.734e-3	3	1825.988	2	NC	1
499		3	max	0	1	.076	3	.014	3	6.187e-3	2	NC	5	NC	2
500			min	0	3	-.136	2	-.005	10	-4.443e-3	3	993.381	2	6145.598	1
501		4	max	0	1	.094	3	.02	1	7.215e-3	2	NC	5	NC	2
502			min	0	3	-.169	2	-.005	10	-5.152e-3	3	759.462	2	4351.329	1
503		5	max	0	1	.103	3	.023	1	8.243e-3	2	NC	5	NC	2
504			min	0	3	-.184	2	-.006	10	-5.861e-3	3	690.415	2	3978.168	1
505		6	max	0	1	.102	3	.023	3	9.271e-3	2	NC	5	NC	2
506			min	0	3	-.179	2	-.008	10	-6.57e-3	3	712.662	2	4575.58	1
507		7	max	0	1	.094	3	.025	3	1.03e-2	2	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.158	2	-.013	2	-7.278e-3	3	824.094	2	6449.812	3
509	8	max	0	1	.081	3	.025	3	1.133e-2	2	NC	4	NC	1
510		min	0	3	-.129	2	-.02	2	-7.987e-3	3	1061.036	2	6207.41	3
511	9	max	0	1	.068	3	.025	3	1.235e-2	2	NC	4	NC	1
512		min	0	3	-.101	2	-.025	2	-8.696e-3	3	1461.07	2	6296.645	3
513	10	max	0	1	.062	3	.024	3	1.338e-2	2	NC	4	NC	4
514		min	0	3	-.088	2	-.028	2	-9.405e-3	3	1771.813	2	5539.192	2
515	11	max	0	1	.068	3	.023	3	1.235e-2	2	NC	4	NC	1
516		min	0	3	-.101	2	-.025	2	-8.694e-3	3	1461.07	2	6362.824	2
517	12	max	0	1	.081	3	.021	3	1.133e-2	2	NC	4	NC	1
518		min	0	3	-.129	2	-.02	2	-7.984e-3	3	1061.036	2	8055.4	3
519	13	max	0	1	.094	3	.02	3	1.03e-2	2	NC	5	NC	2
520		min	0	3	-.158	2	-.013	2	-7.273e-3	3	824.094	2	7288.115	1
521	14	max	0	1	.102	3	.019	1	9.271e-3	2	NC	5	NC	2
522		min	0	3	-.179	2	-.008	10	-6.562e-3	3	712.662	2	4582.163	1
523	15	max	0	1	.103	3	.023	1	8.244e-3	2	NC	5	NC	2
524		min	0	3	-.184	2	-.006	10	-5.852e-3	3	690.415	2	3990.71	1
525	16	max	0	1	.094	3	.02	1	7.216e-3	2	NC	5	NC	2
526		min	0	3	-.169	2	-.005	10	-5.141e-3	3	759.462	2	4373.012	1
527	17	max	0	1	.076	3	.013	1	6.188e-3	2	NC	5	NC	2
528		min	0	3	-.136	2	-.005	10	-4.43e-3	3	993.381	2	6192.006	1
529	18	max	0	1	.05	3	.009	3	5.161e-3	2	NC	4	NC	1
530		min	0	3	-.086	2	-.006	2	-3.719e-3	3	1825.988	2	NC	1
531	19	max	0	1	.02	3	.008	3	4.133e-3	2	NC	1	NC	1
532		min	0	3	-.027	2	-.008	2	-3.009e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	3.912e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.943e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	8.152e-4	3	NC	1	NC	1
536		min	0	2	-.004	4	0	3	-4.708e-4	2	NC	1	NC	1
537	3	max	0	3	-.002	15	.003	1	1.239e-3	3	NC	1	NC	1
538		min	0	2	-.008	4	-.003	3	-8.922e-4	2	8800.218	4	NC	1
539	4	max	0	3	-.003	15	.006	1	1.663e-3	3	NC	3	NC	4
540		min	0	2	-.011	4	-.007	3	-1.314e-3	2	6037.463	4	5571.118	3
541	5	max	0	3	-.003	15	.009	2	2.087e-3	3	NC	5	NC	4
542		min	0	2	-.014	4	-.011	3	-1.735e-3	2	4711.094	4	3668.846	3
543	6	max	0	3	-.004	15	.014	2	2.512e-3	3	NC	5	NC	4
544		min	-.001	2	-.017	4	-.017	3	-2.156e-3	2	3964.884	4	2677.612	3
545	7	max	0	3	-.005	15	.018	2	2.936e-3	3	NC	15	NC	4
546		min	-.001	2	-.019	4	-.022	3	-2.578e-3	2	3516.136	4	2096.734	3
547	8	max	0	3	-.005	15	.022	2	3.36e-3	3	NC	15	NC	4
548		min	-.001	2	-.021	4	-.027	3	-2.999e-3	2	3246.821	4	1731.032	3
549	9	max	0	3	-.005	15	.026	2	3.784e-3	3	NC	15	NC	4
550		min	-.002	2	-.022	4	-.031	3	-3.42e-3	2	3101.859	4	1491.49	3
551	10	max	0	3	-.005	15	.028	2	4.208e-3	3	NC	15	NC	4
552		min	-.002	2	-.022	4	-.035	3	-3.842e-3	2	3056	4	1333.329	3
553	11	max	0	3	-.005	15	.03	2	4.632e-3	3	NC	15	NC	4
554		min	-.002	2	-.022	4	-.037	3	-4.263e-3	2	3101.859	4	1233.021	3
555	12	max	.001	3	-.005	15	.03	2	5.056e-3	3	NC	15	NC	5
556		min	-.002	2	-.021	4	-.038	3	-4.685e-3	2	3246.821	4	1178.911	3
557	13	max	.001	3	-.005	15	.029	2	5.48e-3	3	NC	15	NC	5
558		min	-.002	2	-.019	4	-.037	3	-5.106e-3	2	3516.136	4	1167.671	3
559	14	max	.001	3	-.004	15	.027	1	5.904e-3	3	NC	5	NC	4
560		min	-.003	2	-.017	4	-.034	3	-5.527e-3	2	3964.884	4	1204.47	3
561	15	max	.001	3	-.003	15	.022	1	6.328e-3	3	NC	5	NC	4
562		min	-.003	2	-.015	4	-.028	3	-5.949e-3	2	4711.094	4	1308.087	3
563	16	max	.001	3	-.003	15	.016	1	6.752e-3	3	NC	3	NC	4
564		min	-.003	2	-.012	4	-.018	3	-6.37e-3	2	6037.463	4	1529.429	3



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™
Software
Version 2.4.5673.0

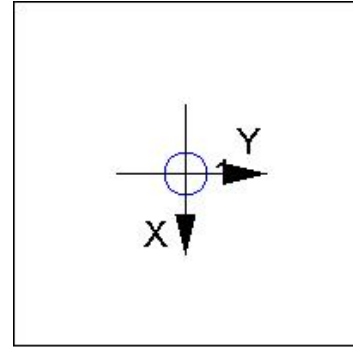
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

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Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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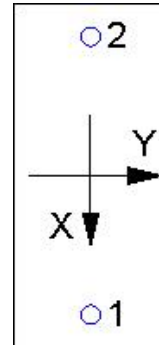
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
 Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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