

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	15° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	85 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 11.34 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	126 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.067 k-ft
$M_z$ =	0.280 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>99%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.162 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.183 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>92%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.599 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>13%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.924 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>13%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>48.30</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	18.93 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.573 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	18.592 k
Utilization =	<u>20%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

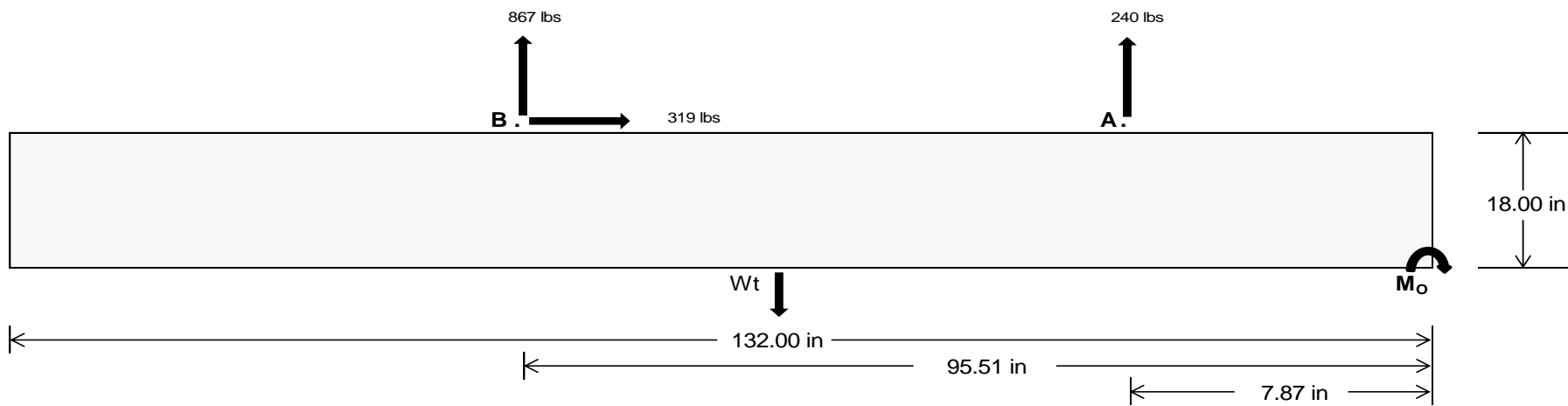
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>	
Tensile Load =	<u>1009.75</u>	<u>3620.31</u>	k
Compressive Load =	<u>4679.02</u>	<u>4857.44</u>	k
Lateral Load =	<u>11.05</u>	<u>1328.44</u>	k
Moment (Weak Axis) =	<u>0.02</u>	<u>0.01</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 90442.3$  in-lbs  
Resisting Force Required = 1370.34 lbs  
S.F. = 1.67  
Weight Required = 2283.90 lbs  
Minimum Width = 21 in  
Weight Provided = 4186.88 lbs

### Sliding

Force = 319.12 lbs  
Friction = 0.4  
Weight Required = 797.81 lbs  
Resisting Weight = 4186.88 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 319.12 lbs  
Cohesion = 130 psf  
Area = 19.25 ft<sup>2</sup>  
Resisting = 2093.44 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 132in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

$$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) = \begin{matrix} \text{Ballast Width} \\ \hline \begin{matrix} 21 \text{ in} & 22 \text{ in} & 23 \text{ in} & 24 \text{ in} \end{matrix} \\ \hline \begin{matrix} 4187 \text{ lbs} & 4386 \text{ lbs} & 4586 \text{ lbs} & 4785 \text{ lbs} \end{matrix} \end{matrix}$$

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	1810 lbs	1810 lbs	1810 lbs	1810 lbs	1282 lbs	1282 lbs	1282 lbs	1282 lbs	2186 lbs	2186 lbs	2186 lbs	2186 lbs	-479 lbs	-479 lbs	-479 lbs	-479 lbs
$F_B$	1877 lbs	1877 lbs	1877 lbs	1877 lbs	1331 lbs	1331 lbs	1331 lbs	1331 lbs	2267 lbs	2267 lbs	2267 lbs	2267 lbs	-1734 lbs	-1734 lbs	-1734 lbs	-1734 lbs
$F_V$	162 lbs	162 lbs	162 lbs	162 lbs	575 lbs	575 lbs	575 lbs	575 lbs	542 lbs	542 lbs	542 lbs	542 lbs	-638 lbs	-638 lbs	-638 lbs	-638 lbs
$P_{total}$	7874 lbs	8074 lbs	8273 lbs	8473 lbs	6800 lbs	7000 lbs	7199 lbs	7398 lbs	8640 lbs	8839 lbs	9038 lbs	9238 lbs	299 lbs	419 lbs	538 lbs	658 lbs
$M$	4396 lbs-ft	4396 lbs-ft	4396 lbs-ft	4396 lbs-ft	3802 lbs-ft	3802 lbs-ft	3802 lbs-ft	3802 lbs-ft	5824 lbs-ft	5824 lbs-ft	5824 lbs-ft	5824 lbs-ft	987 lbs-ft	987 lbs-ft	987 lbs-ft	987 lbs-ft
$e$	0.56 ft	0.54 ft	0.53 ft	0.52 ft	0.56 ft	0.54 ft	0.53 ft	0.51 ft	0.67 ft	0.66 ft	0.64 ft	0.63 ft	3.30 ft	2.36 ft	1.83 ft	1.50 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	284.5 psf	281.4 psf	278.7 psf	276.1 psf	245.5 psf	244.2 psf	243.1 psf	242.0 psf	283.8 psf	280.8 psf	278.0 psf	275.5 psf	0.0 psf	0.0 psf	0.0 psf	5.4 psf
$f_{max}$	533.6 psf	519.3 psf	506.1 psf	494.1 psf	461.0 psf	449.9 psf	439.8 psf	430.6 psf	613.8 psf	595.8 psf	579.4 psf	564.3 psf	51.8 psf	48.4 psf	51.1 psf	54.4 psf

Maximum Bearing Pressure = 614 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

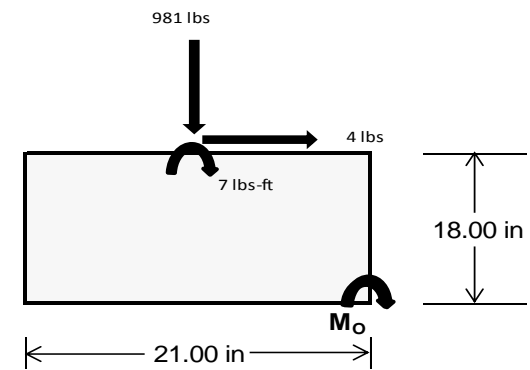
#### Overturning Check

$M_o = 845.6 \text{ ft-lbs}$   
 Resisting Force Required = 966.42 lbs  
 S.F. = 1.67  
 Weight Required = 1610.70 lbs  
 Minimum Width = 21 in  
 Weight Provided = 4186.88 lbs

*A minimum 132in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_y$	247 lbs	676 lbs	247 lbs	981 lbs	3000 lbs	981 lbs	72 lbs	198 lbs	72 lbs
$F_v$	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	5430 lbs	4187 lbs	5430 lbs	5915 lbs	4187 lbs	5915 lbs	1588 lbs	4187 lbs	1588 lbs
$M$	3 lbs-ft	0 lbs-ft	3 lbs-ft	13 lbs-ft	0 lbs-ft	13 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft	0.29 ft
$f_{min}$	281.5 psf	217.5 psf	281.5 psf	305.0 psf	217.5 psf	305.0 psf	82.4 psf	217.5 psf	82.4 psf
$f_{max}$	282.7 psf	217.5 psf	282.7 psf	309.6 psf	217.5 psf	309.6 psf	82.5 psf	217.5 psf	82.5 psf



Maximum Bearing Pressure = 310 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

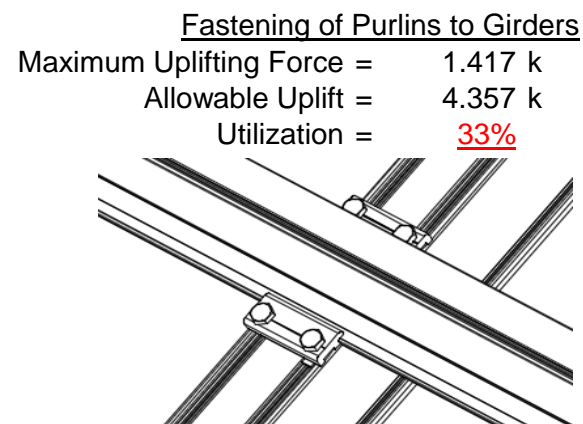
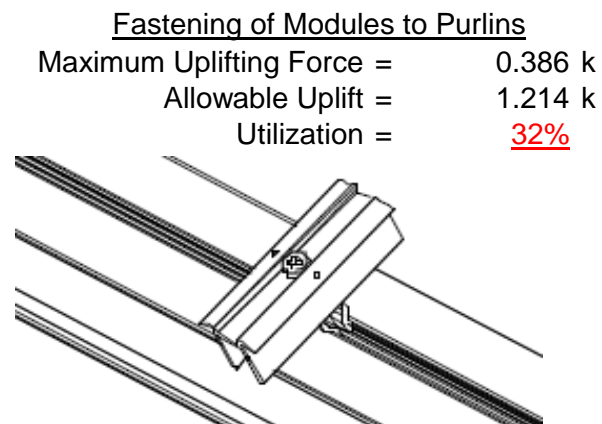
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

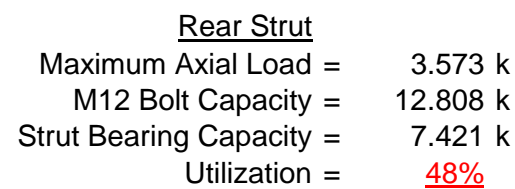
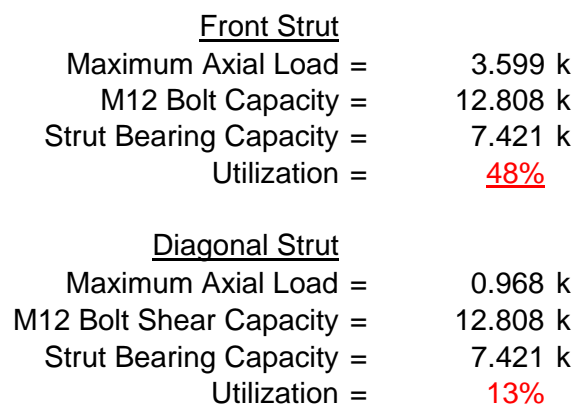
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



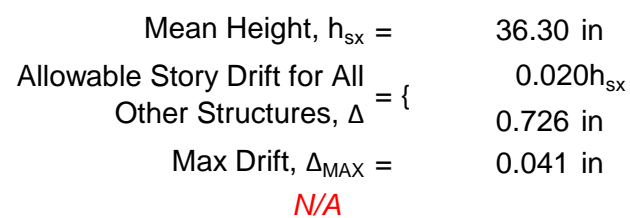
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 126 \text{ in}$$

$$J = 0.432$$

$$348.575$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 126$$

$$J = 0.432$$

$$221.673$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-31.635	-31.635	0	0
2	M14	y	-31.635	-31.635	0	0
3	M15	y	-50.616	-50.616	0	0
4	M16	y	-50.616	-50.616	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	72.761	72.761	0	0
2	M14	y	56.31	56.31	0	0
3	M15	y	31.635	31.635	0	0
4	M16	y	31.635	31.635	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 15° 85mph 30psf 10.5ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	93.506	1	253.622	1	-461	12	.013	1	-.004	15	.662	3
28			min	3.06	15	-210.998	3	-22.061	1	0	3	-.132	1	-.752	1
29		15	max	93.506	1	100.594	1	11.031	1	.013	1	-.004	12	.832	3
30			min	3.06	15	-81.217	3	.364	15	0	3	-.138	1	-.959	1
31		16	max	93.506	1	48.565	3	44.124	1	.013	1	-.003	12	.851	3
32			min	3.06	15	-52.435	1	1.443	15	0	3	-.106	1	-.987	1
33		17	max	93.506	1	178.347	3	77.216	1	.013	1	0	12	.719	3
34			min	3.06	15	-205.464	1	2.522	15	0	3	-.035	1	-.836	1
35		18	max	93.506	1	308.129	3	110.309	1	.013	1	.074	1	.435	3
36			min	3.06	15	-358.492	1	3.601	15	0	3	.002	15	-.507	1
37		19	max	93.506	1	437.91	3	143.401	1	.013	1	.222	1	0	1
38			min	3.06	15	-511.521	1	4.68	15	0	3	.007	15	0	3
39	M14	1	max	42.336	1	537.433	1	-4.819	15	.005	3	.252	1	0	1
40			min	1.389	15	-342.887	3	-147.66	1	-.011	1	.008	15	0	3
41		2	max	42.336	1	384.405	1	-3.74	15	.005	3	.099	1	.342	3
42			min	1.389	15	-243.814	3	-114.568	1	-.011	1	.003	15	-.538	1
43		3	max	42.336	1	231.376	1	-2.661	15	.005	3	0	3	.569	3
44			min	1.389	15	-144.741	3	-81.475	1	-.011	1	-.015	1	-.897	1
45		4	max	42.336	1	78.347	1	-1.582	15	.005	3	-.003	12	.68	3
46			min	1.389	15	-45.668	3	-48.383	1	-.011	1	-.091	1	-1.078	1
47		5	max	42.336	1	53.405	3	-.503	15	.005	3	-.004	12	.675	3
48			min	1.389	15	-74.681	1	-15.29	1	-.011	1	-.128	1	-1.08	1
49		6	max	42.336	1	152.478	3	17.802	1	.005	3	-.004	15	.555	3
50			min	1.389	15	-227.71	1	.326	12	-.011	1	-.127	1	-.903	1
51		7	max	42.336	1	251.552	3	50.894	1	.005	3	-.003	15	.32	3
52			min	1.389	15	-380.739	1	1.405	12	-.011	1	-.087	1	-.548	1
53		8	max	42.336	1	350.625	3	83.987	1	.005	3	0	10	0	15
54			min	1.389	15	-533.767	1	2.483	12	-.011	1	-.008	1	-.032	3
55		9	max	42.336	1	449.698	3	117.079	1	.005	3	.109	1	.697	1
56			min	1.389	15	-686.796	1	3.562	12	-.011	1	.003	12	-.498	3
57		10	max	42.336	1	548.771	3	150.172	1	.005	3	.265	1	1.588	1
58			min	1.389	15	-839.825	1	4.641	12	-.011	1	.007	12	-1.081	3
59		11	max	42.336	1	686.796	1	-3.562	12	.011	1	.109	1	.697	1
60			min	1.389	15	-449.698	3	-117.079	1	-.005	3	.003	12	-.498	3
61		12	max	42.336	1	533.767	1	-2.483	12	.011	1	0	10	0	15
62			min	1.389	15	-350.625	3	-83.987	1	-.005	3	-.008	1	-.032	3
63		13	max	42.336	1	380.739	1	-1.405	12	.011	1	-.003	15	.32	3
64			min	1.389	15	-251.552	3	-50.894	1	-.005	3	-.087	1	-.548	1
65		14	max	42.336	1	227.71	1	-.326	12	.011	1	-.004	15	.555	3
66			min	1.389	15	-152.478	3	-17.802	1	-.005	3	-.127	1	-.903	1
67		15	max	42.336	1	74.681	1	15.29	1	.011	1	-.004	12	.675	3
68			min	1.389	15	-53.405	3	.503	15	-.005	3	-.128	1	-1.08	1
69		16	max	42.336	1	45.668	3	48.383	1	.011	1	-.003	12	.68	3
70			min	1.389	15	-78.347	1	1.582	15	-.005	3	-.091	1	-1.078	1
71		17	max	42.336	1	144.741	3	81.475	1	.011	1	0	3	.569	3
72			min	1.389	15	-231.376	1	2.661	15	-.005	3	-.015	1	-.897	1
73		18	max	42.336	1	243.814	3	114.568	1	.011	1	.099	1	.342	3
74			min	1.389	15	-384.405	1	3.74	15	-.005	3	.003	15	-.538	1
75		19	max	42.336	1	342.887	3	147.66	1	.011	1	.252	1	0	1
76			min	1.389	15	-537.433	1	4.819	15	-.005	3	.008	15	0	3
77	M15	1	max	-1.461	15	597.212	1	-4.818	15	.011	1	.252	1	0	1
78			min	-44.548	1	-187.576	3	-147.638	1	-.004	3	.008	15	0	3
79		2	max	-1.461	15	426.468	1	-3.739	15	.011	1	.099	1	.188	3
80			min	-44.548	1	-134.563	3	-114.545	1	-.004	3	.003	15	-.597	1
81		3	max	-1.461	15	255.724	1	-2.66	15	.011	1	0	3	.314	3
82			min	-44.548	1	-81.55	3	-81.453	1	-.004	3	-.015	1	-.995	1
83		4	max	-1.461	15	84.979	1	-1.581	15	.011	1	-.003	12	.378	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-44.548	1	-28.537	3	-48.36	1	-.004	3	-.091	1	-1.194	1
85		5	max	-1.461	15	24.477	3	-.502	15	.011	1	-.004	12	.381	3
86			min	-44.548	1	-85.765	1	-15.268	1	-.004	3	-.128	1	-1.193	1
87		6	max	-1.461	15	77.49	3	17.824	1	.011	1	-.004	15	.321	3
88			min	-44.548	1	-256.509	1	.346	12	-.004	3	-.127	1	-.994	1
89		7	max	-1.461	15	130.503	3	50.917	1	.011	1	-.003	15	.2	3
90			min	-44.548	1	-427.253	1	1.425	12	-.004	3	-.087	1	-.595	1
91		8	max	-1.461	15	183.516	3	84.009	1	.011	1	0	10	.017	3
92			min	-44.548	1	-597.998	1	2.503	12	-.004	3	-.008	1	-.003	9
93		9	max	-1.461	15	236.529	3	117.102	1	.011	1	.109	1	.8	1
94			min	-44.548	1	-768.742	1	3.582	12	-.004	3	.003	12	-.228	3
95		10	max	-1.461	15	289.542	3	150.194	1	.011	1	.265	1	1.797	1
96			min	-44.548	1	-939.486	1	4.661	12	-.004	3	.007	12	-.535	3
97		11	max	-1.461	15	768.742	1	-3.582	12	.004	3	.109	1	.8	1
98			min	-44.548	1	-236.529	3	-117.102	1	-.011	1	.003	12	-.228	3
99		12	max	-1.461	15	597.998	1	-2.503	12	.004	3	0	10	.017	3
100			min	-44.548	1	-183.516	3	-84.009	1	-.011	1	-.008	1	-.003	9
101		13	max	-1.461	15	427.253	1	-1.425	12	.004	3	-.003	15	.2	3
102			min	-44.548	1	-130.503	3	-50.917	1	-.011	1	-.087	1	-.595	1
103		14	max	-1.461	15	256.509	1	-.346	12	.004	3	-.004	15	.321	3
104			min	-44.548	1	-77.49	3	-17.824	1	-.011	1	-.127	1	-.994	1
105		15	max	-1.461	15	85.765	1	15.268	1	.004	3	-.004	12	.381	3
106			min	-44.548	1	-24.477	3	.502	15	-.011	1	-.128	1	-1.193	1
107		16	max	-1.461	15	28.537	3	48.36	1	.004	3	-.003	12	.378	3
108			min	-44.548	1	-84.979	1	1.581	15	-.011	1	-.091	1	-1.194	1
109		17	max	-1.461	15	81.55	3	81.453	1	.004	3	0	3	.314	3
110			min	-44.548	1	-255.724	1	2.66	15	-.011	1	-.015	1	-.995	1
111		18	max	-1.461	15	134.563	3	114.545	1	.004	3	.099	1	.188	3
112			min	-44.548	1	-426.468	1	3.739	15	-.011	1	.003	15	-.597	1
113		19	max	-1.461	15	187.576	3	147.638	1	.004	3	.252	1	0	1
114			min	-44.548	1	-597.212	1	4.818	15	-.011	1	.008	15	0	3
115	M16	1	max	-3.233	15	571.5	1	-4.684	15	.012	1	.223	1	0	1
116			min	-98.626	1	-176.502	3	-143.567	1	-.006	3	.007	15	0	3
117		2	max	-3.233	15	400.755	1	-3.605	15	.012	1	.075	1	.175	3
118			min	-98.626	1	-123.489	3	-110.474	1	-.006	3	.002	15	-.567	1
119		3	max	-3.233	15	230.011	1	-2.527	15	.012	1	0	12	.288	3
120			min	-98.626	1	-70.476	3	-77.382	1	-.006	3	-.034	1	-.935	1
121		4	max	-3.233	15	59.267	1	-1.448	15	.012	1	-.003	12	.339	3
122			min	-98.626	1	-17.463	3	-44.289	1	-.006	3	-.105	1	-1.104	1
123		5	max	-3.233	15	35.55	3	-.369	15	.012	1	-.004	12	.329	3
124			min	-98.626	1	-111.477	1	-11.197	1	-.006	3	-.138	1	-1.073	1
125		6	max	-3.233	15	88.563	3	21.895	1	.012	1	-.004	15	.256	3
126			min	-98.626	1	-282.222	1	.524	12	-.006	3	-.131	1	-.844	1
127		7	max	-3.233	15	141.577	3	54.988	1	.012	1	-.003	15	.122	3
128			min	-98.626	1	-452.966	1	1.603	12	-.006	3	-.087	1	-.415	1
129		8	max	-3.233	15	194.59	3	88.08	1	.012	1	0	10	.213	1
130			min	-98.626	1	-623.71	1	2.682	12	-.006	3	-.003	1	-.074	3
131		9	max	-3.233	15	247.603	3	121.173	1	.012	1	.119	1	1.04	1
132			min	-98.626	1	-794.455	1	3.761	12	-.006	3	.003	12	-.332	3
133		10	max	-3.233	15	300.616	3	154.265	1	.012	1	.28	1	2.067	1
134			min	-98.626	1	-965.199	1	4.839	12	-.006	3	.008	12	-.652	3
135		11	max	-3.233	15	794.455	1	-3.761	12	.006	3	.119	1	1.04	1
136			min	-98.626	1	-247.603	3	-121.173	1	-.012	1	.003	12	-.332	3
137		12	max	-3.233	15	623.71	1	-2.682	12	.006	3	0	10	.213	1
138			min	-98.626	1	-194.59	3	-88.08	1	-.012	1	-.003	1	-.074	3
139		13	max	-3.233	15	452.966	1	-1.603	12	.006	3	-.003	15	.122	3
140			min	-98.626	1	-141.577	3	-54.988	1	-.012	1	-.087	1	-.415	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-3.233	15	282.222	1	-.524	12	.006	3	-.004	15	.256	3
142			min	-98.626	1	-88.563	3	-21.895	1	-.012	1	-.131	1	-.844	1
143		15	max	-3.233	15	111.477	1	11.197	1	.006	3	-.004	12	.329	3
144			min	-98.626	1	-35.55	3	.369	15	-.012	1	-.138	1	-1.073	1
145		16	max	-3.233	15	17.463	3	44.289	1	.006	3	-.003	12	.339	3
146			min	-98.626	1	-59.267	1	1.448	15	-.012	1	-.105	1	-1.104	1
147		17	max	-3.233	15	70.476	3	77.382	1	.006	3	0	12	.288	3
148			min	-98.626	1	-230.011	1	2.527	15	-.012	1	-.034	1	-.935	1
149		18	max	-3.233	15	123.489	3	110.474	1	.006	3	.075	1	.175	3
150			min	-98.626	1	-400.755	1	3.605	15	-.012	1	.002	15	-.567	1
151		19	max	-3.233	15	176.502	3	143.567	1	.006	3	.223	1	0	1
152			min	-98.626	1	-571.5	1	4.684	15	-.012	1	.007	15	0	3
153	M2	1	max	1104.422	1	2.279	4	1.327	1	0	3	0	3	0	1
154			min	-768.721	3	.537	15	.043	15	0	1	0	1	0	1
155		2	max	1104.75	1	2.264	4	1.327	1	0	3	0	1	0	15
156			min	-768.475	3	.533	15	.043	15	0	1	0	15	0	4
157		3	max	1105.079	1	2.249	4	1.327	1	0	3	0	1	0	15
158			min	-768.229	3	.53	15	.043	15	0	1	0	15	-.001	4
159		4	max	1105.407	1	2.233	4	1.327	1	0	3	0	1	0	15
160			min	-767.982	3	.526	15	.043	15	0	1	0	15	-.001	4
161		5	max	1105.735	1	2.218	4	1.327	1	0	3	.001	1	0	15
162			min	-767.736	3	.523	15	.043	15	0	1	0	15	-.002	4
163		6	max	1106.064	1	2.203	4	1.327	1	0	3	.001	1	0	15
164			min	-767.49	3	.519	15	.043	15	0	1	0	15	-.002	4
165		7	max	1106.392	1	2.188	4	1.327	1	0	3	.002	1	0	15
166			min	-767.243	3	.516	15	.043	15	0	1	0	15	-.003	4
167		8	max	1106.721	1	2.172	4	1.327	1	0	3	.002	1	0	15
168			min	-766.997	3	.512	15	.043	15	0	1	0	15	-.003	4
169		9	max	1107.049	1	2.157	4	1.327	1	0	3	.002	1	0	15
170			min	-766.751	3	.508	15	.043	15	0	1	0	15	-.004	4
171		10	max	1107.378	1	2.142	4	1.327	1	0	3	.003	1	-.001	15
172			min	-766.504	3	.505	15	.043	15	0	1	0	15	-.004	4
173		11	max	1107.706	1	2.127	4	1.327	1	0	3	.003	1	-.001	15
174			min	-766.258	3	.501	15	.043	15	0	1	0	15	-.005	4
175		12	max	1108.034	1	2.111	4	1.327	1	0	3	.003	1	-.001	15
176			min	-766.012	3	.498	15	.043	15	0	1	0	15	-.005	4
177		13	max	1108.363	1	2.096	4	1.327	1	0	3	.004	1	-.001	15
178			min	-765.765	3	.494	15	.043	15	0	1	0	15	-.006	4
179		14	max	1108.691	1	2.081	4	1.327	1	0	3	.004	1	-.001	15
180			min	-765.519	3	.49	15	.043	15	0	1	0	15	-.006	4
181		15	max	1109.02	1	2.065	4	1.327	1	0	3	.004	1	-.002	15
182			min	-765.273	3	.487	15	.043	15	0	1	0	15	-.007	4
183		16	max	1109.348	1	2.05	4	1.327	1	0	3	.004	1	-.002	15
184			min	-765.026	3	.483	15	.043	15	0	1	0	15	-.007	4
185		17	max	1109.677	1	2.035	4	1.327	1	0	3	.005	1	-.002	15
186			min	-764.78	3	.48	15	.043	15	0	1	0	15	-.008	4
187		18	max	1110.005	1	2.02	4	1.327	1	0	3	.005	1	-.002	15
188			min	-764.534	3	.476	15	.043	15	0	1	0	15	-.008	4
189		19	max	1110.334	1	2.004	4	1.327	1	0	3	.005	1	-.002	15
190			min	-764.287	3	.473	15	.043	15	0	1	0	15	-.009	4
191	M3	1	max	203.632	2	8.077	4	.011	1	0	3	0	1	.009	4
192			min	-308.86	3	1.899	15	0	15	0	1	0	15	.002	15
193		2	max	203.461	2	7.305	4	.011	1	0	3	0	1	.005	4
194			min	-308.988	3	1.718	15	0	15	0	1	0	15	.001	15
195		3	max	203.291	2	6.532	4	.011	1	0	3	0	1	.003	2
196			min	-309.115	3	1.536	15	0	15	0	1	0	15	0	12
197		4	max	203.121	2	5.76	4	.011	1	0	3	0	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-309.243	3	1.355	15	0	15	0	1	0	15	0	3
199	5	max	202.95	2	4.987	4	.011	1	0	3	0	1	0	15
200		min	-309.371	3	1.173	15	0	15	0	1	0	15	-.002	4
201	6	max	202.78	2	4.215	4	.011	1	0	3	0	1	-.001	15
202		min	-309.499	3	.992	15	0	15	0	1	0	15	-.004	4
203	7	max	202.61	2	3.443	4	.011	1	0	3	0	1	-.001	15
204		min	-309.626	3	.81	15	0	15	0	1	0	15	-.006	4
205	8	max	202.439	2	2.67	4	.011	1	0	3	0	1	-.002	15
206		min	-309.754	3	.628	15	0	15	0	1	0	15	-.007	4
207	9	max	202.269	2	1.898	4	.011	1	0	3	0	1	-.002	15
208		min	-309.882	3	.447	15	0	15	0	1	0	15	-.008	4
209	10	max	202.099	2	1.125	4	.011	1	0	3	0	1	-.002	15
210		min	-310.01	3	.265	15	0	15	0	1	0	15	-.009	4
211	11	max	201.928	2	.371	2	.011	1	0	3	0	1	-.002	15
212		min	-310.137	3	.054	12	0	15	0	1	0	15	-.009	4
213	12	max	201.758	2	-.098	15	.011	1	0	3	0	1	-.002	15
214		min	-310.265	3	-.42	4	0	15	0	1	0	15	-.009	4
215	13	max	201.588	2	-.279	15	.011	1	0	3	0	1	-.002	15
216		min	-310.393	3	-1.192	4	0	15	0	1	0	15	-.009	4
217	14	max	201.417	2	-.461	15	.011	1	0	3	0	1	-.002	15
218		min	-310.521	3	-1.964	4	0	15	0	1	0	15	-.008	4
219	15	max	201.247	2	-.643	15	.011	1	0	3	0	1	-.002	15
220		min	-310.648	3	-2.737	4	0	15	0	1	0	15	-.007	4
221	16	max	201.076	2	-.824	15	.011	1	0	3	0	1	-.001	15
222		min	-310.776	3	-3.509	4	0	15	0	1	0	15	-.006	4
223	17	max	200.906	2	-1.006	15	.011	1	0	3	0	1	-.001	15
224		min	-310.904	3	-4.282	4	0	15	0	1	0	15	-.004	4
225	18	max	200.736	2	-1.187	15	.011	1	0	3	0	1	0	15
226		min	-311.032	3	-5.054	4	0	15	0	1	0	15	-.002	4
227	19	max	200.565	2	-1.369	15	.011	1	0	3	0	1	0	1
228		min	-311.159	3	-5.827	4	0	15	0	1	0	15	0	1
229	M4	1	max	1228.23	1	0	1	-.289	15	0	1	0	1	0
230		min	-222.375	3	0	1	-8.841	1	0	1	0	15	0	1
231	2	max	1228.401	1	0	1	-.289	15	0	1	0	12	0	1
232		min	-222.247	3	0	1	-8.841	1	0	1	0	1	0	1
233	3	max	1228.571	1	0	1	-.289	15	0	1	0	15	0	1
234		min	-222.119	3	0	1	-8.841	1	0	1	-.002	1	0	1
235	4	max	1228.741	1	0	1	-.289	15	0	1	0	15	0	1
236		min	-221.992	3	0	1	-8.841	1	0	1	-.003	1	0	1
237	5	max	1228.912	1	0	1	-.289	15	0	1	0	15	0	1
238		min	-221.864	3	0	1	-8.841	1	0	1	-.004	1	0	1
239	6	max	1229.082	1	0	1	-.289	15	0	1	0	15	0	1
240		min	-221.736	3	0	1	-8.841	1	0	1	-.005	1	0	1
241	7	max	1229.252	1	0	1	-.289	15	0	1	0	15	0	1
242		min	-221.608	3	0	1	-8.841	1	0	1	-.006	1	0	1
243	8	max	1229.423	1	0	1	-.289	15	0	1	0	15	0	1
244		min	-221.481	3	0	1	-8.841	1	0	1	-.007	1	0	1
245	9	max	1229.593	1	0	1	-.289	15	0	1	0	15	0	1
246		min	-221.353	3	0	1	-8.841	1	0	1	-.008	1	0	1
247	10	max	1229.763	1	0	1	-.289	15	0	1	0	15	0	1
248		min	-221.225	3	0	1	-8.841	1	0	1	-.009	1	0	1
249	11	max	1229.934	1	0	1	-.289	15	0	1	0	15	0	1
250		min	-221.097	3	0	1	-8.841	1	0	1	-.01	1	0	1
251	12	max	1230.104	1	0	1	-.289	15	0	1	0	15	0	1
252		min	-220.969	3	0	1	-8.841	1	0	1	-.011	1	0	1
253	13	max	1230.274	1	0	1	-.289	15	0	1	0	15	0	1
254		min	-220.842	3	0	1	-8.841	1	0	1	-.012	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1230.445	1	0	1	-.289	15	0	1	0	15	0	1
256		min	-220.714	3	0	1	-8.841	1	0	1	-.013	1	0	1
257	15	max	1230.615	1	0	1	-.289	15	0	1	0	15	0	1
258		min	-220.586	3	0	1	-8.841	1	0	1	-.014	1	0	1
259	16	max	1230.785	1	0	1	-.289	15	0	1	0	15	0	1
260		min	-220.458	3	0	1	-8.841	1	0	1	-.015	1	0	1
261	17	max	1230.956	1	0	1	-.289	15	0	1	0	15	0	1
262		min	-220.331	3	0	1	-8.841	1	0	1	-.016	1	0	1
263	18	max	1231.126	1	0	1	-.289	15	0	1	0	15	0	1
264		min	-220.203	3	0	1	-8.841	1	0	1	-.017	1	0	1
265	19	max	1231.296	1	0	1	-.289	15	0	1	0	15	0	1
266		min	-220.075	3	0	1	-8.841	1	0	1	-.018	1	0	1
267	M6	1	max	3567.236	1	2.514	2	0	1	0	0	1	0	1
268		min	-2532.188	3	.376	12	0	1	0	1	0	1	0	1
269	2	max	3567.565	1	2.502	2	0	1	0	1	0	1	0	12
270		min	-2531.942	3	.37	12	0	1	0	1	0	1	0	2
271	3	max	3567.893	1	2.49	2	0	1	0	1	0	1	0	12
272		min	-2531.696	3	.364	12	0	1	0	1	0	1	-.001	2
273	4	max	3568.222	1	2.478	2	0	1	0	1	0	1	0	12
274		min	-2531.449	3	.358	12	0	1	0	1	0	1	-.002	2
275	5	max	3568.55	1	2.466	2	0	1	0	1	0	1	0	12
276		min	-2531.203	3	.352	12	0	1	0	1	0	1	-.002	2
277	6	max	3568.879	1	2.454	2	0	1	0	1	0	1	0	12
278		min	-2530.957	3	.346	12	0	1	0	1	0	1	-.003	2
279	7	max	3569.207	1	2.443	2	0	1	0	1	0	1	0	12
280		min	-2530.71	3	.34	12	0	1	0	1	0	1	-.003	2
281	8	max	3569.535	1	2.431	2	0	1	0	1	0	1	0	12
282		min	-2530.464	3	.334	12	0	1	0	1	0	1	-.004	2
283	9	max	3569.864	1	2.419	2	0	1	0	1	0	1	0	12
284		min	-2530.218	3	.328	12	0	1	0	1	0	1	-.004	2
285	10	max	3570.192	1	2.407	2	0	1	0	1	0	1	0	12
286		min	-2529.971	3	.322	12	0	1	0	1	0	1	-.005	2
287	11	max	3570.521	1	2.395	2	0	1	0	1	0	1	0	12
288		min	-2529.725	3	.316	12	0	1	0	1	0	1	-.005	2
289	12	max	3570.849	1	2.383	2	0	1	0	1	0	1	0	12
290		min	-2529.479	3	.31	12	0	1	0	1	0	1	-.006	2
291	13	max	3571.178	1	2.371	2	0	1	0	1	0	1	0	12
292		min	-2529.232	3	.304	12	0	1	0	1	0	1	-.006	2
293	14	max	3571.506	1	2.359	2	0	1	0	1	0	1	0	12
294		min	-2528.986	3	.298	12	0	1	0	1	0	1	-.007	2
295	15	max	3571.835	1	2.347	2	0	1	0	1	0	1	-.001	12
296		min	-2528.74	3	.292	12	0	1	0	1	0	1	-.008	2
297	16	max	3572.163	1	2.336	2	0	1	0	1	0	1	-.001	12
298		min	-2528.493	3	.287	12	0	1	0	1	0	1	-.008	2
299	17	max	3572.491	1	2.324	2	0	1	0	1	0	1	-.001	12
300		min	-2528.247	3	.281	12	0	1	0	1	0	1	-.009	2
301	18	max	3572.82	1	2.312	2	0	1	0	1	0	1	-.001	12
302		min	-2528.001	3	.275	12	0	1	0	1	0	1	-.009	2
303	19	max	3573.148	1	2.3	2	0	1	0	1	0	1	-.001	12
304		min	-2527.755	3	.269	12	0	1	0	1	0	1	-.01	2
305	M7	1	max	923.859	2	8.12	4	0	1	0	0	1	.01	2
306		min	-965.393	3	1.905	15	0	1	0	1	0	1	.001	12
307	2	max	923.688	2	7.347	4	0	1	0	1	0	1	.007	2
308		min	-965.52	3	1.723	15	0	1	0	1	0	1	0	3
309	3	max	923.518	2	6.575	4	0	1	0	1	0	1	.004	2
310		min	-965.648	3	1.542	15	0	1	0	1	0	1	-.002	3
311	4	max	923.348	2	5.802	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-965.776	3	1.36	15	0	1	0	1	0	1	-.003	3
313	5	max	923.177	2	5.03	4	0	1	0	1	0	1	0	2
314		min	-965.904	3	1.179	15	0	1	0	1	0	1	-.004	3
315	6	max	923.007	2	4.258	4	0	1	0	1	0	1	-.001	15
316		min	-966.031	3	.997	15	0	1	0	1	0	1	-.005	3
317	7	max	922.837	2	3.485	4	0	1	0	1	0	1	-.001	15
318		min	-966.159	3	.815	15	0	1	0	1	0	1	-.006	4
319	8	max	922.666	2	2.713	4	0	1	0	1	0	1	-.002	15
320		min	-966.287	3	.634	15	0	1	0	1	0	1	-.007	4
321	9	max	922.496	2	1.94	4	0	1	0	1	0	1	-.002	15
322		min	-966.415	3	.452	15	0	1	0	1	0	1	-.008	4
323	10	max	922.326	2	1.265	2	0	1	0	1	0	1	-.002	15
324		min	-966.543	3	.169	12	0	1	0	1	0	1	-.009	4
325	11	max	922.155	2	.663	2	0	1	0	1	0	1	-.002	15
326		min	-966.67	3	-.225	3	0	1	0	1	0	1	-.009	4
327	12	max	921.985	2	.061	2	0	1	0	1	0	1	-.002	15
328		min	-966.798	3	-.676	3	0	1	0	1	0	1	-.009	4
329	13	max	921.815	2	-.274	15	0	1	0	1	0	1	-.002	15
330		min	-966.926	3	-1.149	4	0	1	0	1	0	1	-.009	4
331	14	max	921.644	2	-.456	15	0	1	0	1	0	1	-.002	15
332		min	-967.054	3	-1.922	4	0	1	0	1	0	1	-.008	4
333	15	max	921.474	2	-.637	15	0	1	0	1	0	1	-.002	15
334		min	-967.181	3	-2.694	4	0	1	0	1	0	1	-.007	4
335	16	max	921.303	2	-.819	15	0	1	0	1	0	1	-.001	15
336		min	-967.309	3	-3.467	4	0	1	0	1	0	1	-.006	4
337	17	max	921.133	2	-.1	15	0	1	0	1	0	1	0	15
338		min	-967.437	3	-4.239	4	0	1	0	1	0	1	-.004	4
339	18	max	920.963	2	-1.182	15	0	1	0	1	0	1	0	15
340		min	-967.565	3	-5.012	4	0	1	0	1	0	1	-.002	4
341	19	max	920.792	2	-1.363	15	0	1	0	1	0	1	0	1
342		min	-967.692	3	-5.784	4	0	1	0	1	0	1	0	1
343	M8	1	max	3596.183	1	0	1	0	1	0	1	0	1	1
344		min	-779.028	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3596.353	1	0	1	0	1	0	1	0	1	0	1
346		min	-778.9	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3596.524	1	0	1	0	1	0	1	0	1	0	1
348		min	-778.772	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3596.694	1	0	1	0	1	0	1	0	1	0	1
350		min	-778.645	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3596.864	1	0	1	0	1	0	1	0	1	0	1
352		min	-778.517	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3597.035	1	0	1	0	1	0	1	0	1	0	1
354		min	-778.389	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3597.205	1	0	1	0	1	0	1	0	1	0	1
356		min	-778.261	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3597.376	1	0	1	0	1	0	1	0	1	0	1
358		min	-778.134	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3597.546	1	0	1	0	1	0	1	0	1	0	1
360		min	-778.006	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3597.716	1	0	1	0	1	0	1	0	1	0	1
362		min	-777.878	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3597.887	1	0	1	0	1	0	1	0	1	0	1
364		min	-777.75	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3598.057	1	0	1	0	1	0	1	0	1	0	1
366		min	-777.622	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3598.227	1	0	1	0	1	0	1	0	1	0	1
368		min	-777.495	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3598.398	1	0	1	0	1	0	1	0	1	0	1
370			min	-777.367	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3598.568	1	0	1	0	1	0	1	0	1	0	1
372			min	-777.239	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3598.738	1	0	1	0	1	0	1	0	1	0	1
374			min	-777.111	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3598.909	1	0	1	0	1	0	1	0	1	0	1
376			min	-776.984	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3599.079	1	0	1	0	1	0	1	0	1	0	1
378			min	-776.856	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3599.249	1	0	1	0	1	0	1	0	1	0	1
380			min	-776.728	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1104.422	1	2.279	4	-.043	15	0	1	0	1	0	1
382			min	-768.721	3	.537	15	-1.327	1	0	3	0	3	0	1
383		2	max	1104.75	1	2.264	4	-.043	15	0	1	0	15	0	15
384			min	-768.475	3	.533	15	-1.327	1	0	3	0	1	0	4
385		3	max	1105.079	1	2.249	4	-.043	15	0	1	0	15	0	15
386			min	-768.229	3	.53	15	-1.327	1	0	3	0	1	-.001	4
387		4	max	1105.407	1	2.233	4	-.043	15	0	1	0	15	0	15
388			min	-767.982	3	.526	15	-1.327	1	0	3	0	1	-.001	4
389		5	max	1105.735	1	2.218	4	-.043	15	0	1	0	15	0	15
390			min	-767.736	3	.523	15	-1.327	1	0	3	-.001	1	-.002	4
391		6	max	1106.064	1	2.203	4	-.043	15	0	1	0	15	0	15
392			min	-767.49	3	.519	15	-1.327	1	0	3	-.001	1	-.002	4
393		7	max	1106.392	1	2.188	4	-.043	15	0	1	0	15	0	15
394			min	-767.243	3	.516	15	-1.327	1	0	3	-.002	1	-.003	4
395		8	max	1106.721	1	2.172	4	-.043	15	0	1	0	15	0	15
396			min	-766.997	3	.512	15	-1.327	1	0	3	-.002	1	-.003	4
397		9	max	1107.049	1	2.157	4	-.043	15	0	1	0	15	0	15
398			min	-766.751	3	.508	15	-1.327	1	0	3	-.002	1	-.004	4
399		10	max	1107.378	1	2.142	4	-.043	15	0	1	0	15	-.001	15
400			min	-766.504	3	.505	15	-1.327	1	0	3	-.003	1	-.004	4
401		11	max	1107.706	1	2.127	4	-.043	15	0	1	0	15	-.001	15
402			min	-766.258	3	.501	15	-1.327	1	0	3	-.003	1	-.005	4
403		12	max	1108.034	1	2.111	4	-.043	15	0	1	0	15	-.001	15
404			min	-766.012	3	.498	15	-1.327	1	0	3	-.003	1	-.005	4
405		13	max	1108.363	1	2.096	4	-.043	15	0	1	0	15	-.001	15
406			min	-765.765	3	.494	15	-1.327	1	0	3	-.004	1	-.006	4
407		14	max	1108.691	1	2.081	4	-.043	15	0	1	0	15	-.001	15
408			min	-765.519	3	.49	15	-1.327	1	0	3	-.004	1	-.006	4
409		15	max	1109.02	1	2.065	4	-.043	15	0	1	0	15	-.002	15
410			min	-765.273	3	.487	15	-1.327	1	0	3	-.004	1	-.007	4
411		16	max	1109.348	1	2.05	4	-.043	15	0	1	0	15	-.002	15
412			min	-765.026	3	.483	15	-1.327	1	0	3	-.004	1	-.007	4
413		17	max	1109.677	1	2.035	4	-.043	15	0	1	0	15	-.002	15
414			min	-764.78	3	.48	15	-1.327	1	0	3	-.005	1	-.008	4
415		18	max	1110.005	1	2.02	4	-.043	15	0	1	0	15	-.002	15
416			min	-764.534	3	.476	15	-1.327	1	0	3	-.005	1	-.008	4
417		19	max	1110.334	1	2.004	4	-.043	15	0	1	0	15	-.002	15
418			min	-764.287	3	.473	15	-1.327	1	0	3	-.005	1	-.009	4
419	M11	1	max	203.632	2	8.077	4	0	15	0	1	0	15	.009	4
420			min	-308.86	3	1.899	15	-.011	1	0	3	0	1	.002	15
421		2	max	203.461	2	7.305	4	0	15	0	1	0	15	.005	4
422			min	-308.988	3	1.718	15	-.011	1	0	3	0	1	.001	15
423		3	max	203.291	2	6.532	4	0	15	0	1	0	15	.003	2
424			min	-309.115	3	1.536	15	-.011	1	0	3	0	1	0	12
425		4	max	203.121	2	5.76	4	0	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-309.243	3	1.355	15	-.011	1	0	3	0	1	0	3
427		5	max	202.95	2	4.987	4	0	15	0	1	0	15	0	15
428			min	-309.371	3	1.173	15	-.011	1	0	3	0	1	-.002	4
429		6	max	202.78	2	4.215	4	0	15	0	1	0	15	-.001	15
430			min	-309.499	3	.992	15	-.011	1	0	3	0	1	-.004	4
431		7	max	202.61	2	3.443	4	0	15	0	1	0	15	-.001	15
432			min	-309.626	3	.81	15	-.011	1	0	3	0	1	-.006	4
433		8	max	202.439	2	2.67	4	0	15	0	1	0	15	-.002	15
434			min	-309.754	3	.628	15	-.011	1	0	3	0	1	-.007	4
435		9	max	202.269	2	1.898	4	0	15	0	1	0	15	-.002	15
436			min	-309.882	3	.447	15	-.011	1	0	3	0	1	-.008	4
437		10	max	202.099	2	1.125	4	0	15	0	1	0	15	-.002	15
438			min	-310.01	3	.265	15	-.011	1	0	3	0	1	-.009	4
439		11	max	201.928	2	.371	2	0	15	0	1	0	15	-.002	15
440			min	-310.137	3	.054	12	-.011	1	0	3	0	1	-.009	4
441		12	max	201.758	2	-.098	15	0	15	0	1	0	15	-.002	15
442			min	-310.265	3	-.42	4	-.011	1	0	3	0	1	-.009	4
443		13	max	201.588	2	-.279	15	0	15	0	1	0	15	-.002	15
444			min	-310.393	3	-1.192	4	-.011	1	0	3	0	1	-.009	4
445		14	max	201.417	2	-.461	15	0	15	0	1	0	15	-.002	15
446			min	-310.521	3	-1.964	4	-.011	1	0	3	0	1	-.008	4
447		15	max	201.247	2	-.643	15	0	15	0	1	0	15	-.002	15
448			min	-310.648	3	-2.737	4	-.011	1	0	3	0	1	-.007	4
449		16	max	201.076	2	-.824	15	0	15	0	1	0	15	-.001	15
450			min	-310.776	3	-3.509	4	-.011	1	0	3	0	1	-.006	4
451		17	max	200.906	2	-1.006	15	0	15	0	1	0	15	-.001	15
452			min	-310.904	3	-4.282	4	-.011	1	0	3	0	1	-.004	4
453		18	max	200.736	2	-1.187	15	0	15	0	1	0	15	0	15
454			min	-311.032	3	-5.054	4	-.011	1	0	3	0	1	-.002	4
455		19	max	200.565	2	-1.369	15	0	15	0	1	0	15	0	1
456			min	-311.159	3	-5.827	4	-.011	1	0	3	0	1	0	1
457	M12	1	max	1228.23	1	0	1	8.841	1	0	1	0	15	0	1
458			min	-222.375	3	0	1	.289	15	0	1	0	1	0	1
459		2	max	1228.401	1	0	1	8.841	1	0	1	0	1	0	1
460			min	-222.247	3	0	1	.289	15	0	1	0	12	0	1
461		3	max	1228.571	1	0	1	8.841	1	0	1	.002	1	0	1
462			min	-222.119	3	0	1	.289	15	0	1	0	15	0	1
463		4	max	1228.741	1	0	1	8.841	1	0	1	.003	1	0	1
464			min	-221.992	3	0	1	.289	15	0	1	0	15	0	1
465		5	max	1228.912	1	0	1	8.841	1	0	1	.004	1	0	1
466			min	-221.864	3	0	1	.289	15	0	1	0	15	0	1
467		6	max	1229.082	1	0	1	8.841	1	0	1	.005	1	0	1
468			min	-221.736	3	0	1	.289	15	0	1	0	15	0	1
469		7	max	1229.252	1	0	1	8.841	1	0	1	.006	1	0	1
470			min	-221.608	3	0	1	.289	15	0	1	0	15	0	1
471		8	max	1229.423	1	0	1	8.841	1	0	1	.007	1	0	1
472			min	-221.481	3	0	1	.289	15	0	1	0	15	0	1
473		9	max	1229.593	1	0	1	8.841	1	0	1	.008	1	0	1
474			min	-221.353	3	0	1	.289	15	0	1	0	15	0	1
475		10	max	1229.763	1	0	1	8.841	1	0	1	.009	1	0	1
476			min	-221.225	3	0	1	.289	15	0	1	0	15	0	1
477		11	max	1229.934	1	0	1	8.841	1	0	1	.01	1	0	1
478			min	-221.097	3	0	1	.289	15	0	1	0	15	0	1
479		12	max	1230.104	1	0	1	8.841	1	0	1	.011	1	0	1
480			min	-220.969	3	0	1	.289	15	0	1	0	15	0	1
481		13	max	1230.274	1	0	1	8.841	1	0	1	.012	1	0	1
482			min	-220.842	3	0	1	.289	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1230.445	1	0	1	8.841	1	0	1	.013	1	0	1
484		min	-220.714	3	0	1	.289	15	0	1	0	15	0	1
485	15	max	1230.615	1	0	1	8.841	1	0	1	.014	1	0	1
486		min	-220.586	3	0	1	.289	15	0	1	0	15	0	1
487	16	max	1230.785	1	0	1	8.841	1	0	1	.015	1	0	1
488		min	-220.458	3	0	1	.289	15	0	1	0	15	0	1
489	17	max	1230.956	1	0	1	8.841	1	0	1	.016	1	0	1
490		min	-220.331	3	0	1	.289	15	0	1	0	15	0	1
491	18	max	1231.126	1	0	1	8.841	1	0	1	.017	1	0	1
492		min	-220.203	3	0	1	.289	15	0	1	0	15	0	1
493	19	max	1231.296	1	0	1	8.841	1	0	1	.018	1	0	1
494		min	-220.075	3	0	1	.289	15	0	1	0	15	0	1
495	M1	1	max	143.404	1	437.9	3	-3.06	15	0	.222	1	0	3
496		min	4.68	15	-510.323	1	-93.413	1	0	3	.007	15	-.013	1
497	2	max	143.774	1	436.863	3	-3.06	15	0	1	.173	1	.256	1
498		min	4.792	15	-511.707	1	-93.413	1	0	3	.006	15	-.23	3
499	3	max	180.931	3	561.417	1	-3.017	15	0	3	.124	1	.514	1
500		min	-119.868	2	-313.85	3	-92.322	1	0	1	.004	15	-.452	3
501	4	max	181.209	3	560.033	1	-3.017	15	0	3	.075	1	.218	1
502		min	-119.497	2	-314.888	3	-92.322	1	0	1	.002	15	-.286	3
503	5	max	181.487	3	558.649	1	-3.017	15	0	3	.026	1	-.004	15
504		min	-119.126	2	-315.926	3	-92.322	1	0	1	0	15	-.119	3
505	6	max	181.765	3	557.266	1	-3.017	15	0	3	0	15	.048	3
506		min	-118.756	2	-316.963	3	-92.322	1	0	1	-.022	1	-.372	1
507	7	max	182.043	3	555.882	1	-3.017	15	0	3	-.002	15	.215	3
508		min	-118.385	2	-318.001	3	-92.322	1	0	1	-.071	1	-.665	1
509	8	max	182.321	3	554.499	1	-3.017	15	0	3	-.004	15	.383	3
510		min	-118.014	2	-319.039	3	-92.322	1	0	1	-.12	1	-.958	1
511	9	max	190.225	3	30.671	2	-4.406	15	0	9	.07	1	.448	3
512		min	-60.757	2	.42	15	-134.678	1	0	3	.002	15	-1.091	1
513	10	max	190.503	3	29.287	2	-4.406	15	0	9	0	15	.436	3
514		min	-60.386	2	.002	15	-134.678	1	0	3	0	1	-1.1	1
515	11	max	190.781	3	27.903	2	-4.406	15	0	9	-.002	15	.425	3
516		min	-60.015	2	-1.71	4	-134.678	1	0	3	-.072	1	-1.108	1
517	12	max	198.654	3	208.72	3	-2.945	15	0	1	.118	1	.369	3
518		min	-38.197	10	-586.71	1	-90.174	1	0	3	.004	15	-.977	1
519	13	max	198.932	3	207.682	3	-2.945	15	0	1	.071	1	.26	3
520		min	-37.888	10	-588.094	1	-90.174	1	0	3	.002	15	-.667	1
521	14	max	199.21	3	206.645	3	-2.945	15	0	1	.023	1	.15	3
522		min	-37.579	10	-589.477	1	-90.174	1	0	3	0	15	-.357	1
523	15	max	199.488	3	205.607	3	-2.945	15	0	1	0	15	.042	3
524		min	-37.27	10	-590.861	1	-90.174	1	0	3	-.024	1	-.045	1
525	16	max	199.766	3	204.569	3	-2.945	15	0	1	-.002	15	.267	1
526		min	-36.961	10	-592.244	1	-90.174	1	0	3	-.072	1	-.067	3
527	17	max	200.044	3	203.532	3	-2.945	15	0	1	-.004	15	.58	1
528		min	-36.653	10	-593.628	1	-90.174	1	0	3	-.12	1	-.174	3
529	18	max	-4.796	15	574.023	1	-3.233	15	0	3	-.006	15	.29	1
530		min	-143.936	1	-175.489	3	-98.716	1	0	1	-.171	1	-.087	3
531	19	max	-4.684	15	572.639	1	-3.233	15	0	3	-.007	15	.006	3
532		min	-143.565	1	-176.527	3	-98.716	1	0	1	-.223	1	-.012	1
533	M5	1	max	308.857	1	1460.211	3	0	1	0	0	1	.026	1
534		min	9.551	12	-1724.247	1	0	1	0	1	0	1	0	3
535	2	max	309.228	1	1459.173	3	0	1	0	1	0	1	.937	1
536		min	9.737	12	-1725.63	1	0	1	0	1	0	1	-.771	3
537	3	max	581.11	3	1729.448	1	0	1	0	1	0	1	1.806	1
538		min	-486.477	1	-1016.238	3	0	1	0	1	0	1	-1.511	3
539	4	max	581.388	3	1728.064	1	0	1	0	1	0	1	.894	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-486.106	1	-1017.275	3	0	1	0	1	0	1	-.974	3
541		5	max	581.666	3	1726.68	1	0	1	0	1	0	1	.011	9
542			min	-485.736	1	-1018.313	3	0	1	0	1	0	1	-.437	3
543		6	max	581.944	3	1725.297	1	0	1	0	1	0	1	.1	3
544			min	-485.365	1	-1019.351	3	0	1	0	1	0	1	-.928	1
545		7	max	582.222	3	1723.913	1	0	1	0	1	0	1	.638	3
546			min	-484.994	1	-1020.389	3	0	1	0	1	0	1	-1.838	1
547		8	max	582.5	3	1722.529	1	0	1	0	1	0	1	1.177	3
548			min	-484.623	1	-1021.426	3	0	1	0	1	0	1	-2.748	1
549		9	max	596.651	3	101.129	2	0	1	0	1	0	1	1.356	3
550			min	-330.305	2	.418	15	0	1	0	1	0	1	-3.106	1
551		10	max	596.929	3	99.745	2	0	1	0	1	0	1	1.313	3
552			min	-329.934	2	.001	15	0	1	0	1	0	1	-3.134	1
553		11	max	597.207	3	98.362	2	0	1	0	1	0	1	1.271	3
554			min	-329.563	2	-1.576	4	0	1	0	1	0	1	-3.162	1
555		12	max	611.422	3	664.43	3	0	1	0	1	0	1	1.115	3
556			min	-212.569	2	-1831.182	1	0	1	0	1	0	1	-2.816	1
557		13	max	611.7	3	663.393	3	0	1	0	1	0	1	.765	3
558			min	-212.198	2	-1832.566	1	0	1	0	1	0	1	-1.849	1
559		14	max	611.978	3	662.355	3	0	1	0	1	0	1	.415	3
560			min	-211.828	2	-1833.95	1	0	1	0	1	0	1	-.882	1
561		15	max	612.256	3	661.317	3	0	1	0	1	0	1	.126	2
562			min	-211.457	2	-1835.333	1	0	1	0	1	0	1	-.004	13
563		16	max	612.534	3	660.279	3	0	1	0	1	0	1	1.055	1
564			min	-211.086	2	-1836.717	1	0	1	0	1	0	1	-.283	3
565		17	max	612.812	3	659.242	3	0	1	0	1	0	1	2.025	1
566			min	-210.715	2	-1838.1	1	0	1	0	1	0	1	-.631	3
567		18	max	-9.864	12	1938.606	1	0	1	0	1	0	1	1.047	1
568			min	-308.905	1	-600.347	3	0	1	0	1	0	1	-.33	3
569		19	max	-9.678	12	1937.222	1	0	1	0	1	0	1	.024	1
570			min	-308.534	1	-601.385	3	0	1	0	1	0	1	-.013	3
571	M9	1	max	143.404	1	437.9	3	93.413	1	0	3	-.007	15	0	3
572			min	4.68	15	-510.323	1	3.06	15	0	1	-.222	1	-.013	1
573		2	max	143.774	1	436.863	3	93.413	1	0	3	-.006	15	.256	1
574			min	4.792	15	-511.707	1	3.06	15	0	1	-.173	1	-.23	3
575		3	max	180.931	3	561.417	1	92.322	1	0	1	-.004	15	.514	1
576			min	-119.868	2	-313.85	3	3.017	15	0	3	-.124	1	-.452	3
577		4	max	181.209	3	560.033	1	92.322	1	0	1	-.002	15	.218	1
578			min	-119.497	2	-314.888	3	3.017	15	0	3	-.075	1	-.286	3
579		5	max	181.487	3	558.649	1	92.322	1	0	1	0	15	-.004	15
580			min	-119.126	2	-315.926	3	3.017	15	0	3	-.026	1	-.119	3
581		6	max	181.765	3	557.266	1	92.322	1	0	1	.022	1	.048	3
582			min	-118.756	2	-316.963	3	3.017	15	0	3	0	15	-.372	1
583		7	max	182.043	3	555.882	1	92.322	1	0	1	.071	1	.215	3
584			min	-118.385	2	-318.001	3	3.017	15	0	3	.002	15	-.665	1
585		8	max	182.321	3	554.499	1	92.322	1	0	1	.12	1	.383	3
586			min	-118.014	2	-319.039	3	3.017	15	0	3	.004	15	-.958	1
587		9	max	190.225	3	30.671	2	134.678	1	0	3	-.002	15	.448	3
588			min	-60.757	2	.42	15	4.406	15	0	9	-.07	1	-1.091	1
589		10	max	190.503	3	29.287	2	134.678	1	0	3	0	1	.436	3
590			min	-60.386	2	.002	15	4.406	15	0	9	0	15	-1.1	1
591		11	max	190.781	3	27.903	2	134.678	1	0	3	.072	1	.425	3
592			min	-60.015	2	-1.71	4	4.406	15	0	9	.002	15	-1.108	1
593		12	max	198.654	3	208.72	3	90.174	1	0	3	-.004	15	.369	3
594			min	-38.197	10	-586.71	1	2.945	15	0	1	-.118	1	-.977	1
595		13	max	198.932	3	207.682	3	90.174	1	0	3	-.002	15	.26	3
596			min	-37.888	10	-588.094	1	2.945	15	0	1	-.071	1	-.667	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	199.21	3	206.645	3	90.174	1	0	3	0	15	.15	3
598		min	-37.579	10	-589.477	1	2.945	15	0	1	-.023	1	-.357	1
599	15	max	199.488	3	205.607	3	90.174	1	0	3	.024	1	.042	3
600		min	-37.27	10	-590.861	1	2.945	15	0	1	0	15	-.045	1
601	16	max	199.766	3	204.569	3	90.174	1	0	3	.072	1	.267	1
602		min	-36.961	10	-592.244	1	2.945	15	0	1	.002	15	-.067	3
603	17	max	200.044	3	203.532	3	90.174	1	0	3	.12	1	.58	1
604		min	-36.653	10	-593.628	1	2.945	15	0	1	.004	15	-.174	3
605	18	max	-4.796	15	574.023	1	98.716	1	0	1	.171	1	.29	1
606		min	-143.936	1	-175.489	3	3.233	15	0	3	.006	15	-.087	3
607	19	max	-4.684	15	572.639	1	98.716	1	0	1	.223	1	.006	3
608		min	-143.565	1	-176.527	3	3.233	15	0	3	.007	15	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.11	1	.003	3	8.698e-3	1	NC	1	NC	1
2			min	0	15	-.012	3	-.001	10	-9.101e-4	3	NC	1	NC	1
3		2	max	0	1	.227	3	.039	1	1.006e-2	1	NC	5	NC	2
4			min	0	15	-.147	1	.001	15	-9.534e-4	3	980.05	1	6895.987	1
5		3	max	0	1	.42	3	.092	1	1.141e-2	1	NC	5	NC	3
6			min	0	15	-.351	1	.003	15	-9.967e-4	3	546.577	1	2802.35	1
7		4	max	0	1	.537	3	.139	1	1.277e-2	1	NC	5	NC	3
8			min	0	15	-.467	1	.005	15	-1.04e-3	3	437.324	1	1847.454	1
9		5	max	0	1	.563	3	.163	1	1.413e-2	1	NC	5	NC	3
10			min	0	15	-.477	1	.005	15	-1.083e-3	3	429.724	1	1568.923	1
11		6	max	0	1	.501	3	.158	1	1.548e-2	1	NC	5	NC	3
12			min	0	15	-.385	1	.005	15	-1.127e-3	3	490.914	3	1619.65	1
13		7	max	0	1	.369	3	.125	1	1.684e-2	1	NC	5	NC	3
14			min	0	15	-.214	1	.004	15	-1.17e-3	3	661.33	3	2053.645	1
15		8	max	0	1	.201	3	.074	1	1.82e-2	1	NC	5	NC	2
16			min	0	15	-.012	9	.003	15	-1.213e-3	3	1180.79	3	3512.019	1
17		9	max	0	1	.182	1	.023	1	1.955e-2	1	NC	4	NC	1
18			min	0	15	.005	15	-.002	10	-1.256e-3	3	3462.624	1	NC	1
19	10	max	0	1	.266	1	.01	3	2.091e-2	1	NC	3	NC	1	
20		min	0	1	-.019	3	-.006	2	-1.3e-3	3	1611.746	1	NC	1	
21	11	max	0	15	.182	1	.023	1	1.955e-2	1	NC	4	NC	1	
22		min	0	1	.005	15	-.002	10	-1.256e-3	3	3462.624	1	NC	1	
23	12	max	0	15	.201	3	.074	1	1.82e-2	1	NC	5	NC	2	
24		min	0	1	-.012	9	.003	15	-1.213e-3	3	1180.79	3	3512.019	1	
25	13	max	0	15	.369	3	.125	1	1.684e-2	1	NC	5	NC	3	
26		min	0	1	-.214	1	.004	15	-1.17e-3	3	661.33	3	2053.645	1	
27	14	max	0	15	.501	3	.158	1	1.548e-2	1	NC	5	NC	3	
28		min	0	1	-.385	1	.005	15	-1.127e-3	3	490.914	3	1619.65	1	
29	15	max	0	15	.563	3	.163	1	1.413e-2	1	NC	5	NC	3	
30		min	0	1	-.477	1	.005	15	-1.083e-3	3	429.724	1	1568.923	1	
31	16	max	0	15	.537	3	.139	1	1.277e-2	1	NC	5	NC	3	
32		min	0	1	-.467	1	.005	15	-1.04e-3	3	437.324	1	1847.454	1	
33	17	max	0	15	.42	3	.092	1	1.141e-2	1	NC	5	NC	3	
34		min	0	1	-.351	1	.003	15	-9.967e-4	3	546.577	1	2802.35	1	
35	18	max	0	15	.227	3	.039	1	1.006e-2	1	NC	5	NC	2	
36		min	0	1	-.147	1	.001	15	-9.534e-4	3	980.05	1	6895.987	1	
37	19	max	0	15	.11	1	.003	3	8.698e-3	1	NC	1	NC	1	
38		min	0	1	-.012	3	-.001	10	-9.101e-4	3	NC	1	NC	1	
39	M14	1	max	0	1	.131	3	.003	3	5.509e-3	1	NC	1	NC	1
40			min	0	15	-.357	1	0	10	-2.395e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.357	3	.027	1	6.64e-3	1	NC	5	NC	1
42		min	0	15	-.745	1	0	15	-2.928e-3	3	647.963	1	NC	1
43	3	max	0	1	.548	3	.075	1	7.77e-3	1	NC	15	NC	3
44		min	0	15	-1.078	1	.002	15	-3.462e-3	3	349.346	1	3474.788	1
45	4	max	0	1	.677	3	.12	1	8.9e-3	1	NC	15	NC	3
46		min	0	15	-1.315	1	.004	15	-3.996e-3	3	262.907	1	2148.465	1
47	5	max	0	1	.735	3	.145	1	1.003e-2	1	9016.974	15	NC	3
48		min	0	15	-1.437	1	.005	15	-4.529e-3	3	233.141	1	1761.947	1
49	6	max	0	1	.72	3	.144	1	1.116e-2	1	8986.467	15	NC	3
50		min	0	15	-1.445	1	.005	15	-5.063e-3	3	231.564	1	1778.995	1
51	7	max	0	1	.647	3	.116	1	1.229e-2	1	9832.877	15	NC	3
52		min	0	15	-1.356	1	.004	15	-5.597e-3	3	252.089	1	2220.28	1
53	8	max	0	1	.54	3	.07	1	1.342e-2	1	NC	15	NC	2
54		min	0	15	-1.21	1	.002	15	-6.13e-3	3	295.285	1	3746.704	1
55	9	max	0	1	.437	3	.022	1	1.455e-2	1	NC	15	NC	1
56		min	0	15	-1.064	1	-.002	10	-6.664e-3	3	356.349	1	NC	1
57	10	max	0	1	.389	3	.009	3	1.568e-2	1	NC	5	NC	1
58		min	0	1	-.994	1	-.005	2	-7.198e-3	3	395.094	1	NC	1
59	11	max	0	15	.437	3	.022	1	1.455e-2	1	NC	15	NC	1
60		min	0	1	-1.064	1	-.002	10	-6.664e-3	3	356.349	1	NC	1
61	12	max	0	15	.54	3	.07	1	1.342e-2	1	NC	15	NC	2
62		min	0	1	-1.21	1	.002	15	-6.13e-3	3	295.285	1	3746.704	1
63	13	max	0	15	.647	3	.116	1	1.229e-2	1	9832.877	15	NC	3
64		min	0	1	-1.356	1	.004	15	-5.597e-3	3	252.089	1	2220.28	1
65	14	max	0	15	.72	3	.144	1	1.116e-2	1	8986.467	15	NC	3
66		min	0	1	-1.445	1	.005	15	-5.063e-3	3	231.564	1	1778.995	1
67	15	max	0	15	.735	3	.145	1	1.003e-2	1	9016.974	15	NC	3
68		min	0	1	-1.437	1	.005	15	-4.529e-3	3	233.141	1	1761.947	1
69	16	max	0	15	.677	3	.12	1	8.9e-3	1	NC	15	NC	3
70		min	0	1	-1.315	1	.004	15	-3.996e-3	3	262.907	1	2148.465	1
71	17	max	0	15	.548	3	.075	1	7.77e-3	1	NC	15	NC	3
72		min	0	1	-1.078	1	.002	15	-3.462e-3	3	349.346	1	3474.788	1
73	18	max	0	15	.357	3	.027	1	6.64e-3	1	NC	5	NC	1
74		min	0	1	-.745	1	0	15	-2.928e-3	3	647.963	1	NC	1
75	19	max	0	15	.131	3	.003	3	5.509e-3	1	NC	1	NC	1
76		min	0	1	-.357	1	0	10	-2.395e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.134	.003	3	2.012e-3	3	NC	1	NC	1
78		min	0	1	-.356	1	0	10	-5.598e-3	1	NC	1	NC	1
79	2	max	0	15	.276	3	.027	1	2.463e-3	3	NC	5	NC	2
80		min	0	1	-.778	1	0	15	-6.751e-3	1	597.815	1	9974.43	1
81	3	max	0	15	.398	3	.075	1	2.914e-3	3	NC	15	NC	3
82		min	0	1	-1.136	1	.002	15	-7.904e-3	1	322.926	1	3465.654	1
83	4	max	0	15	.487	3	.12	1	3.365e-3	3	NC	15	NC	3
84		min	0	1	-1.39	1	.004	15	-9.057e-3	1	243.829	1	2144.069	1
85	5	max	0	15	.536	3	.145	1	3.816e-3	3	9024.385	15	NC	3
86		min	0	1	-1.516	1	.005	15	-1.021e-2	1	217.335	1	1758.669	1
87	6	max	0	15	.543	3	.144	1	4.266e-3	3	8995.305	15	NC	3
88		min	0	1	-1.515	1	.005	15	-1.136e-2	1	217.544	1	1775.546	1
89	7	max	0	15	.517	3	.116	1	4.717e-3	3	9844.938	15	NC	3
90		min	0	1	-1.408	1	.004	15	-1.252e-2	1	239.604	1	2214.974	1
91	8	max	0	15	.47	3	.07	1	5.168e-3	3	NC	15	NC	2
92		min	0	1	-1.239	1	.002	15	-1.367e-2	1	285.512	1	3732.138	1
93	9	max	0	15	.421	3	.022	1	5.619e-3	3	NC	15	NC	1
94		min	0	1	-1.072	1	-.001	10	-1.482e-2	1	352.124	1	NC	1
95	10	max	0	1	.398	3	.009	3	6.07e-3	3	NC	5	NC	1
96		min	0	1	-.993	1	-.005	2	-1.598e-2	1	395.641	1	NC	1
97	11	max	0	1	.421	3	.022	1	5.619e-3	3	NC	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.072	1	-.001	10	-1.482e-2	1	352.124	1	NC	1
99		12	max	0	1	.47	3	.07	1	5.168e-3	3	NC	15	NC	2
100			min	0	15	-1.239	1	.002	15	-1.367e-2	1	285.512	1	3732.138	1
101		13	max	0	1	.517	3	.116	1	4.717e-3	3	9844.938	15	NC	3
102			min	0	15	-1.408	1	.004	15	-1.252e-2	1	239.604	1	2214.974	1
103		14	max	0	1	.543	3	.144	1	4.266e-3	3	8995.305	15	NC	3
104			min	0	15	-1.515	1	.005	15	-1.136e-2	1	217.544	1	1775.546	1
105		15	max	0	1	.536	3	.145	1	3.816e-3	3	9024.385	15	NC	3
106			min	0	15	-1.516	1	.005	15	-1.021e-2	1	217.335	1	1758.669	1
107		16	max	0	1	.487	3	.12	1	3.365e-3	3	NC	15	NC	3
108			min	0	15	-1.39	1	.004	15	-9.057e-3	1	243.829	1	2144.069	1
109		17	max	0	1	.398	3	.075	1	2.914e-3	3	NC	15	NC	3
110			min	0	15	-1.136	1	.002	15	-7.904e-3	1	322.926	1	3465.654	1
111		18	max	0	1	.276	3	.027	1	2.463e-3	3	NC	5	NC	2
112			min	0	15	-.778	1	0	15	-6.751e-3	1	597.815	1	9974.43	1
113		19	max	0	1	.134	3	.003	3	2.012e-3	3	NC	1	NC	1
114			min	0	15	-.356	1	0	10	-5.598e-3	1	NC	1	NC	1
115	M16	1	max	0	15	.107	1	.002	3	3.48e-3	3	NC	1	NC	1
116			min	0	1	-.044	3	0	10	-8.234e-3	1	NC	1	NC	1
117		2	max	0	15	.041	3	.038	1	4.125e-3	3	NC	5	NC	2
118			min	0	1	-.184	1	.001	15	-9.477e-3	1	866.665	1	6937.669	1
119		3	max	0	15	.108	3	.092	1	4.769e-3	3	NC	5	NC	3
120			min	0	1	-.415	1	.003	15	-1.072e-2	1	482.688	1	2810.355	1
121		4	max	0	15	.145	3	.138	1	5.414e-3	3	NC	5	NC	3
122			min	0	1	-.547	1	.005	15	-1.196e-2	1	385.217	1	1849.511	1
123		5	max	0	15	.146	3	.163	1	6.059e-3	3	NC	5	NC	3
124			min	0	1	-.562	1	.005	15	-1.321e-2	1	376.77	1	1568.417	1
125		6	max	0	15	.111	3	.158	1	6.704e-3	3	NC	5	NC	3
126			min	0	1	-.463	1	.005	15	-1.445e-2	1	442.596	1	1616.404	1
127		7	max	0	15	.049	3	.125	1	7.349e-3	3	NC	5	NC	3
128			min	0	1	-.274	1	.004	15	-1.569e-2	1	662.189	1	2043.845	1
129		8	max	0	15	.001	13	.075	1	7.993e-3	3	NC	3	NC	3
130			min	0	1	-.062	2	.003	15	-1.694e-2	1	1694.376	1	3471.501	1
131		9	max	0	15	.165	1	.024	1	8.638e-3	3	NC	4	NC	1
132			min	0	1	-.091	3	0	10	-1.818e-2	1	4323.985	1	NC	1
133		10	max	0	1	.258	1	.008	3	9.283e-3	3	NC	5	NC	1
134			min	0	1	-.12	3	-.005	2	-1.942e-2	1	1667.642	1	NC	1
135		11	max	0	1	.165	1	.024	1	8.638e-3	3	NC	4	NC	1
136			min	0	15	-.091	3	0	10	-1.818e-2	1	4323.985	1	NC	1
137		12	max	0	1	.001	13	.075	1	7.993e-3	3	NC	3	NC	3
138			min	0	15	-.062	2	.003	15	-1.694e-2	1	1694.376	1	3471.501	1
139		13	max	0	1	.049	3	.125	1	7.349e-3	3	NC	5	NC	3
140			min	0	15	-.274	1	.004	15	-1.569e-2	1	662.189	1	2043.845	1
141		14	max	0	1	.111	3	.158	1	6.704e-3	3	NC	5	NC	3
142			min	0	15	-.463	1	.005	15	-1.445e-2	1	442.596	1	1616.404	1
143		15	max	0	1	.146	3	.163	1	6.059e-3	3	NC	5	NC	3
144			min	0	15	-.562	1	.005	15	-1.321e-2	1	376.77	1	1568.417	1
145		16	max	0	1	.145	3	.138	1	5.414e-3	3	NC	5	NC	3
146			min	0	15	-.547	1	.005	15	-1.196e-2	1	385.217	1	1849.511	1
147		17	max	0	1	.108	3	.092	1	4.769e-3	3	NC	5	NC	3
148			min	0	15	-.415	1	.003	15	-1.072e-2	1	482.688	1	2810.355	1
149		18	max	0	1	.041	3	.038	1	4.125e-3	3	NC	5	NC	2
150			min	0	15	-.184	1	.001	15	-9.477e-3	1	866.665	1	6937.669	1
151		19	max	0	1	.107	1	.002	3	3.48e-3	3	NC	1	NC	1
152			min	0	15	-.044	3	0	10	-8.234e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.002	2	.007	1	-6.063e-6	15	NC	1	NC	2
154			min	-.004	3	-.004	3	0	15	-1.858e-4	1	NC	1	6691.78	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.005	1	.001	2	.007	1	-5.618e-6	15	NC	1	NC	2
156			min	-.003	3	-.004	3	0	15	-1.721e-4	1	NC	1	7297.645	1
157		3	max	.005	1	.001	2	.006	1	-5.173e-6	15	NC	1	NC	2
158			min	-.003	3	-.004	3	0	15	-1.585e-4	1	NC	1	8019.496	1
159		4	max	.004	1	0	2	.005	1	-4.728e-6	15	NC	1	NC	2
160			min	-.003	3	-.004	3	0	15	-1.449e-4	1	NC	1	8888.03	1
161		5	max	.004	1	0	2	.005	1	-4.283e-6	15	NC	1	NC	2
162			min	-.003	3	-.004	3	0	15	-1.312e-4	1	NC	1	9945.122	1
163		6	max	.004	1	0	2	.004	1	-3.838e-6	15	NC	1	NC	1
164			min	-.003	3	-.004	3	0	15	-1.176e-4	1	NC	1	NC	1
165		7	max	.003	1	0	10	.004	1	-3.393e-6	15	NC	1	NC	1
166			min	-.002	3	-.004	3	0	15	-1.039e-4	1	NC	1	NC	1
167		8	max	.003	1	0	10	.003	1	-2.948e-6	15	NC	1	NC	1
168			min	-.002	3	-.004	3	0	15	-9.028e-5	1	NC	1	NC	1
169		9	max	.003	1	0	15	.003	1	-2.503e-6	15	NC	1	NC	1
170			min	-.002	3	-.003	3	0	15	-7.664e-5	1	NC	1	NC	1
171		10	max	.003	1	0	15	.002	1	-2.058e-6	15	NC	1	NC	1
172			min	-.002	3	-.003	3	0	15	-6.299e-5	1	NC	1	NC	1
173		11	max	.002	1	0	15	.002	1	-1.613e-6	15	NC	1	NC	1
174			min	-.002	3	-.003	3	0	15	-4.935e-5	1	NC	1	NC	1
175		12	max	.002	1	0	15	.001	1	-1.168e-6	15	NC	1	NC	1
176			min	-.001	3	-.003	3	0	15	-3.571e-5	1	NC	1	NC	1
177		13	max	.002	1	0	15	.001	1	-7.23e-7	15	NC	1	NC	1
178			min	-.001	3	-.002	3	0	15	-2.206e-5	1	NC	1	NC	1
179		14	max	.001	1	0	15	0	1	-2.779e-7	15	NC	1	NC	1
180			min	0	3	-.002	4	0	15	-8.417e-6	1	NC	1	NC	1
181		15	max	.001	1	0	15	0	1	5.227e-6	1	NC	1	NC	1
182			min	0	3	-.002	4	0	15	0	3	NC	1	NC	1
183		16	max	0	1	0	15	0	1	1.887e-5	1	NC	1	NC	1
184			min	0	3	-.002	4	0	15	5.516e-7	12	NC	1	NC	1
185		17	max	0	1	0	15	0	1	3.252e-5	1	NC	1	NC	1
186			min	0	3	-.001	4	0	15	1.057e-6	15	NC	1	NC	1
187		18	max	0	1	0	15	0	1	4.616e-5	1	NC	1	NC	1
188			min	0	3	0	4	0	15	1.502e-6	15	NC	1	NC	1
189		19	max	0	1	0	1	0	1	5.98e-5	1	NC	1	NC	1
190			min	0	1	0	1	0	1	1.947e-6	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-6.033e-7	15	NC	1	NC	1
192			min	0	1	0	1	0	1	-1.852e-5	1	NC	1	NC	1
193		2	max	0	3	0	15	0	1	4.357e-6	1	NC	1	NC	1
194			min	0	2	-.001	4	0	15	1.427e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	1	2.724e-5	1	NC	1	NC	1
196			min	0	2	-.003	4	0	15	8.887e-7	15	NC	1	NC	1
197		4	max	0	3	-.001	15	0	1	5.012e-5	1	NC	1	NC	1
198			min	0	2	-.005	4	0	15	1.635e-6	15	NC	1	NC	1
199		5	max	0	3	-.002	15	.001	1	7.299e-5	1	NC	1	NC	1
200			min	0	2	-.007	4	0	15	2.381e-6	15	NC	1	NC	1
201		6	max	0	3	-.002	15	.002	1	9.587e-5	1	NC	1	NC	1
202			min	0	2	-.009	4	0	15	3.127e-6	15	NC	1	NC	1
203		7	max	0	3	-.002	15	.002	1	1.188e-4	1	NC	1	NC	1
204			min	0	2	-.01	4	0	15	3.873e-6	15	9281.999	4	NC	1
205		8	max	.001	3	-.003	15	.002	1	1.416e-4	1	NC	1	NC	1
206			min	0	2	-.011	4	0	15	4.619e-6	15	8274.232	4	NC	1
207		9	max	.001	3	-.003	15	.003	1	1.645e-4	1	NC	1	NC	1
208			min	0	2	-.012	4	0	15	5.365e-6	15	7672.049	4	NC	1
209		10	max	.001	3	-.003	15	.003	1	1.874e-4	1	NC	2	NC	1
210			min	0	2	-.013	4	0	15	6.111e-6	15	7368.451	4	NC	1
211		11	max	.002	3	-.003	15	.003	1	2.103e-4	1	NC	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	0	2	-.013	4	0	15	6.857e-6	15	7317.921	4	NC	1
213		max	.002	3	-.003	15	.004	1	2.331e-4	1	NC	2	NC	1
214		min	-.001	2	-.013	4	0	15	7.603e-6	15	7518.842	4	NC	1
215		max	.002	3	-.003	15	.004	1	2.56e-4	1	NC	1	NC	1
216		min	-.001	2	-.012	4	0	15	8.349e-6	15	8015.014	4	NC	1
217		max	.002	3	-.002	15	.005	1	2.789e-4	1	NC	1	NC	1
218		min	-.001	2	-.011	4	0	15	9.095e-6	15	8919.004	4	NC	1
219		max	.002	3	-.002	15	.005	1	3.018e-4	1	NC	1	NC	1
220		min	-.001	2	-.009	4	0	15	9.841e-6	15	NC	1	NC	1
221		max	.002	3	-.002	15	.005	1	3.247e-4	1	NC	1	NC	1
222		min	-.001	2	-.008	1	0	15	1.059e-5	15	NC	1	NC	1
223		max	.002	3	-.001	15	.006	1	3.475e-4	1	NC	1	NC	1
224		min	-.002	2	-.006	1	0	15	1.133e-5	15	NC	1	NC	1
225		max	.003	3	0	15	.006	1	3.704e-4	1	NC	1	NC	1
226		min	-.002	2	-.005	1	0	15	1.208e-5	15	NC	1	NC	1
227		max	.003	3	0	15	.007	1	3.933e-4	1	NC	1	NC	1
228		min	-.002	2	-.003	1	0	15	1.282e-5	15	NC	1	NC	1
229	M4	max	.003	1	.001	2	0	15	-4.626e-7	12	NC	1	NC	3
230		min	0	3	-.003	3	-.007	1	-1.536e-5	1	NC	1	3629.64	1
231		max	.003	1	.001	2	0	15	-4.626e-7	12	NC	1	NC	2
232		min	0	3	-.003	3	-.006	1	-1.536e-5	1	NC	1	3953.646	1
233		max	.003	1	.001	2	0	15	-4.626e-7	12	NC	1	NC	2
234		min	0	3	-.002	3	-.006	1	-1.536e-5	1	NC	1	4338.897	1
235		max	.002	1	.001	2	0	15	-4.626e-7	12	NC	1	NC	2
236		min	0	3	-.002	3	-.005	1	-1.536e-5	1	NC	1	4801.37	1
237		max	.002	1	0	2	0	15	-4.626e-7	12	NC	1	NC	2
238		min	0	3	-.002	3	-.005	1	-1.536e-5	1	NC	1	5362.774	1
239		max	.002	1	0	2	0	15	-4.626e-7	12	NC	1	NC	2
240		min	0	3	-.002	3	-.004	1	-1.536e-5	1	NC	1	6053.218	1
241		max	.002	1	0	2	0	15	-4.626e-7	12	NC	1	NC	2
242		min	0	3	-.002	3	-.004	1	-1.536e-5	1	NC	1	6915.441	1
243		max	.002	1	0	2	0	15	-4.626e-7	12	NC	1	NC	2
244		min	0	3	-.002	3	-.003	1	-1.536e-5	1	NC	1	8011.786	1
245		max	.002	1	0	2	0	15	-4.626e-7	12	NC	1	NC	2
246		min	0	3	-.001	3	-.003	1	-1.536e-5	1	NC	1	9436.124	1
247		max	.001	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
248		min	0	3	-.001	3	-.002	1	-1.536e-5	1	NC	1	NC	1
249		max	.001	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
250		min	0	3	-.001	3	-.002	1	-1.536e-5	1	NC	1	NC	1
251		max	.001	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
252		min	0	3	-.001	3	-.001	1	-1.536e-5	1	NC	1	NC	1
253		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
254		min	0	3	0	3	-.001	1	-1.536e-5	1	NC	1	NC	1
255		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
256		min	0	3	0	3	0	1	-1.536e-5	1	NC	1	NC	1
257		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
258		min	0	3	0	3	0	1	-1.536e-5	1	NC	1	NC	1
259		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
260		min	0	3	0	3	0	1	-1.536e-5	1	NC	1	NC	1
261		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
262		min	0	3	0	3	0	1	-1.536e-5	1	NC	1	NC	1
263		max	0	1	0	2	0	15	-4.626e-7	12	NC	1	NC	1
264		min	0	3	0	3	0	1	-1.536e-5	1	NC	1	NC	1
265		max	0	1	0	1	0	1	-4.626e-7	12	NC	1	NC	1
266		min	0	1	0	1	0	1	-1.536e-5	1	NC	1	NC	1
267	M6	max	.016	1	.009	2	0	1	0	1	NC	3	NC	1
268		min	-.012	3	-.013	3	0	1	0	1	5272.686	2	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.016	1	.008	2	0	1	0	1	NC	1	NC	1
270		min	-.011	3	-.013	3	0	1	0	1	5833.098	2	NC	1
271	3	max	.015	1	.007	2	0	1	0	1	NC	1	NC	1
272		min	-.01	3	-.012	3	0	1	0	1	6520.615	2	NC	1
273	4	max	.014	1	.006	2	0	1	0	1	NC	1	NC	1
274		min	-.01	3	-.011	3	0	1	0	1	7376.027	2	NC	1
275	5	max	.013	1	.006	2	0	1	0	1	NC	1	NC	1
276		min	-.009	3	-.011	3	0	1	0	1	8458.566	2	NC	1
277	6	max	.012	1	.005	2	0	1	0	1	NC	1	NC	1
278		min	-.008	3	-.01	3	0	1	0	1	9857.05	2	NC	1
279	7	max	.011	1	.004	2	0	1	0	1	NC	1	NC	1
280		min	-.008	3	-.009	3	0	1	0	1	NC	1	NC	1
281	8	max	.01	1	.003	2	0	1	0	1	NC	1	NC	1
282		min	-.007	3	-.009	3	0	1	0	1	NC	1	NC	1
283	9	max	.009	1	.003	2	0	1	0	1	NC	1	NC	1
284		min	-.006	3	-.008	3	0	1	0	1	NC	1	NC	1
285	10	max	.008	1	.002	2	0	1	0	1	NC	1	NC	1
286		min	-.006	3	-.007	3	0	1	0	1	NC	1	NC	1
287	11	max	.007	1	.001	2	0	1	0	1	NC	1	NC	1
288		min	-.005	3	-.006	3	0	1	0	1	NC	1	NC	1
289	12	max	.006	1	0	2	0	1	0	1	NC	1	NC	1
290		min	-.005	3	-.006	3	0	1	0	1	NC	1	NC	1
291	13	max	.005	1	0	2	0	1	0	1	NC	1	NC	1
292		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
293	14	max	.005	1	0	2	0	1	0	1	NC	1	NC	1
294		min	-.003	3	-.004	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.003	3	-.003	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
301	18	max	0	1	0	15	0	1	0	1	NC	1	NC	1
302		min	0	3	0	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.002	3	0	1	0	1	NC	1	NC	1
309	3	max	0	3	0	15	0	1	0	1	NC	1	NC	1
310		min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
311	4	max	.001	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.001	2	-.005	3	0	1	0	1	NC	1	NC	1
313	5	max	.002	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.002	2	-.007	4	0	1	0	1	NC	1	NC	1
315	6	max	.002	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.002	2	-.009	4	0	1	0	1	NC	1	NC	1
317	7	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
318		min	-.003	2	-.01	4	0	1	0	1	9568.412	4	NC	1
319	8	max	.003	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.003	2	-.011	4	0	1	0	1	8508.952	4	NC	1
321	9	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.004	2	-.012	4	0	1	0	1	7874.079	4	NC	1
323	10	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.004	2	-.013	4	0	1	0	1	7550.1	4	NC	1
325	11	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.004	2	-.013	4	0	1	0	1	7488.089	4	NC	1
327		12	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.005	2	-.013	4	0	1	0	1	7684.907	4	NC	1
329		13	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.005	2	-.012	4	0	1	0	1	8184.246	4	NC	1
331		14	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.006	2	-.012	1	0	1	0	1	9100.164	4	NC	1
333		15	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.006	2	-.011	1	0	1	0	1	NC	1	NC	1
335		16	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.007	2	-.011	1	0	1	0	1	NC	1	NC	1
337		17	max	.008	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.007	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.008	3	0	15	0	1	0	1	NC	1	NC	1
340			min	-.008	2	-.009	1	0	1	0	1	NC	1	NC	1
341		19	max	.008	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.008	2	-.007	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.007	2	0	1	0	1	NC	1	NC	1
344			min	-.002	3	-.008	3	0	1	0	1	NC	1	NC	1
345		2	max	.008	1	.006	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.008	3	0	1	0	1	NC	1	NC	1
347		3	max	.008	1	.006	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.007	3	0	1	0	1	NC	1	NC	1
349		4	max	.007	1	.005	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.007	3	0	1	0	1	NC	1	NC	1
351		5	max	.007	1	.005	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.006	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	1	.005	2	0	1	0	1	NC	1	NC	1
354			min	-.001	3	-.006	3	0	1	0	1	NC	1	NC	1
355		7	max	.006	1	.004	2	0	1	0	1	NC	1	NC	1
356			min	-.001	3	-.006	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
358			min	-.001	3	-.005	3	0	1	0	1	NC	1	NC	1
359		9	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
360			min	-.001	3	-.005	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.003	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
363		11	max	.004	1	.003	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	1	.002	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.001	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	0	2	0	1	0	1	NC	1	NC	1
376			min	0	3	0	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.005	1	.002	2	0	15	1.858e-4	1	NC	1	NC	2
382			min	-.004	3	-.004	3	-.007	1	6.063e-6	15	NC	1	6691.78	1



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.005	1	.001	2	0	15	1.721e-4	1	NC	1	NC	2
384			min	-.003	3	-.004	3	-.007	1	5.618e-6	15	NC	1	7297.645	1
385		3	max	.005	1	.001	2	0	15	1.585e-4	1	NC	1	NC	2
386			min	-.003	3	-.004	3	-.006	1	5.173e-6	15	NC	1	8019.496	1
387		4	max	.004	1	0	2	0	15	1.449e-4	1	NC	1	NC	2
388			min	-.003	3	-.004	3	-.005	1	4.728e-6	15	NC	1	8888.03	1
389		5	max	.004	1	0	2	0	15	1.312e-4	1	NC	1	NC	2
390			min	-.003	3	-.004	3	-.005	1	4.283e-6	15	NC	1	9945.122	1
391		6	max	.004	1	0	2	0	15	1.176e-4	1	NC	1	NC	1
392			min	-.003	3	-.004	3	-.004	1	3.838e-6	15	NC	1	NC	1
393		7	max	.003	1	0	10	0	15	1.039e-4	1	NC	1	NC	1
394			min	-.002	3	-.004	3	-.004	1	3.393e-6	15	NC	1	NC	1
395		8	max	.003	1	0	10	0	15	9.028e-5	1	NC	1	NC	1
396			min	-.002	3	-.004	3	-.003	1	2.948e-6	15	NC	1	NC	1
397		9	max	.003	1	0	15	0	15	7.664e-5	1	NC	1	NC	1
398			min	-.002	3	-.003	3	-.003	1	2.503e-6	15	NC	1	NC	1
399		10	max	.003	1	0	15	0	15	6.299e-5	1	NC	1	NC	1
400			min	-.002	3	-.003	3	-.002	1	2.058e-6	15	NC	1	NC	1
401		11	max	.002	1	0	15	0	15	4.935e-5	1	NC	1	NC	1
402			min	-.002	3	-.003	3	-.002	1	1.613e-6	15	NC	1	NC	1
403		12	max	.002	1	0	15	0	15	3.571e-5	1	NC	1	NC	1
404			min	-.001	3	-.003	3	-.001	1	1.168e-6	15	NC	1	NC	1
405		13	max	.002	1	0	15	0	15	2.206e-5	1	NC	1	NC	1
406			min	-.001	3	-.002	3	-.001	1	7.23e-7	15	NC	1	NC	1
407		14	max	.001	1	0	15	0	15	8.417e-6	1	NC	1	NC	1
408			min	0	3	-.002	4	0	1	2.779e-7	15	NC	1	NC	1
409		15	max	.001	1	0	15	0	15	0	3	NC	1	NC	1
410			min	0	3	-.002	4	0	1	-5.227e-6	1	NC	1	NC	1
411		16	max	0	1	0	15	0	15	-5.516e-7	12	NC	1	NC	1
412			min	0	3	-.002	4	0	1	-1.887e-5	1	NC	1	NC	1
413		17	max	0	1	0	15	0	15	-1.057e-6	15	NC	1	NC	1
414			min	0	3	-.001	4	0	1	-3.252e-5	1	NC	1	NC	1
415		18	max	0	1	0	15	0	15	-1.502e-6	15	NC	1	NC	1
416			min	0	3	0	4	0	1	-4.616e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	-1.947e-6	15	NC	1	NC	1
418			min	0	1	0	1	0	1	-5.98e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.852e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	6.033e-7	15	NC	1	NC	1
421		2	max	0	3	0	15	0	15	-1.427e-7	15	NC	1	NC	1
422			min	0	2	-.001	4	0	1	-4.357e-6	1	NC	1	NC	1
423		3	max	0	3	0	15	0	15	-8.887e-7	15	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.724e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	0	15	-1.635e-6	15	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-5.012e-5	1	NC	1	NC	1
427		5	max	0	3	-.002	15	0	15	-2.381e-6	15	NC	1	NC	1
428			min	0	2	-.007	4	-.001	1	-7.299e-5	1	NC	1	NC	1
429		6	max	0	3	-.002	15	0	15	-3.127e-6	15	NC	1	NC	1
430			min	0	2	-.009	4	-.002	1	-9.587e-5	1	NC	1	NC	1
431		7	max	0	3	-.002	15	0	15	-3.873e-6	15	NC	1	NC	1
432			min	0	2	-.01	4	-.002	1	-1.188e-4	1	9281.999	4	NC	1
433		8	max	.001	3	-.003	15	0	15	-4.619e-6	15	NC	1	NC	1
434			min	0	2	-.011	4	-.002	1	-1.416e-4	1	8274.232	4	NC	1
435		9	max	.001	3	-.003	15	0	15	-5.365e-6	15	NC	1	NC	1
436			min	0	2	-.012	4	-.003	1	-1.645e-4	1	7672.049	4	NC	1
437		10	max	.001	3	-.003	15	0	15	-6.111e-6	15	NC	2	NC	1
438			min	0	2	-.013	4	-.003	1	-1.874e-4	1	7368.451	4	NC	1
439		11	max	.002	3	-.003	15	0	15	-6.857e-6	15	NC	2	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	0	2	-.013	4	-.003	1	-2.103e-4	1	7317.921	4	NC	1
441		max	.002	3	-.003	15	0	15	-7.603e-6	15	NC	2	NC	1
442		min	-.001	2	-.013	4	-.004	1	-2.331e-4	1	7518.842	4	NC	1
443		max	.002	3	-.003	15	0	15	-8.349e-6	15	NC	1	NC	1
444		min	-.001	2	-.012	4	-.004	1	-2.56e-4	1	8015.014	4	NC	1
445		max	.002	3	-.002	15	0	15	-9.095e-6	15	NC	1	NC	1
446		min	-.001	2	-.011	4	-.005	1	-2.789e-4	1	8919.004	4	NC	1
447		max	.002	3	-.002	15	0	15	-9.841e-6	15	NC	1	NC	1
448		min	-.001	2	-.009	4	-.005	1	-3.018e-4	1	NC	1	NC	1
449		max	.002	3	-.002	15	0	15	-1.059e-5	15	NC	1	NC	1
450		min	-.001	2	-.008	1	-.005	1	-3.247e-4	1	NC	1	NC	1
451		max	.002	3	-.001	15	0	15	-1.133e-5	15	NC	1	NC	1
452		min	-.002	2	-.006	1	-.006	1	-3.475e-4	1	NC	1	NC	1
453		max	.003	3	0	15	0	15	-1.208e-5	15	NC	1	NC	1
454		min	-.002	2	-.005	1	-.006	1	-3.704e-4	1	NC	1	NC	1
455		max	.003	3	0	15	0	15	-1.282e-5	15	NC	1	NC	1
456		min	-.002	2	-.003	1	-.007	1	-3.933e-4	1	NC	1	NC	1
457	M12	max	.003	1	.001	2	.007	1	1.536e-5	1	NC	1	NC	3
458		min	0	3	-.003	3	0	15	4.626e-7	12	NC	1	3629.64	1
459		max	.003	1	.001	2	.006	1	1.536e-5	1	NC	1	NC	2
460		min	0	3	-.003	3	0	15	4.626e-7	12	NC	1	3953.646	1
461		max	.003	1	.001	2	.006	1	1.536e-5	1	NC	1	NC	2
462		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	4338.897	1
463		max	.002	1	.001	2	.005	1	1.536e-5	1	NC	1	NC	2
464		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	4801.37	1
465		max	.002	1	0	2	.005	1	1.536e-5	1	NC	1	NC	2
466		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	5362.774	1
467		max	.002	1	0	2	.004	1	1.536e-5	1	NC	1	NC	2
468		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	6053.218	1
469		max	.002	1	0	2	.004	1	1.536e-5	1	NC	1	NC	2
470		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	6915.441	1
471		max	.002	1	0	2	.003	1	1.536e-5	1	NC	1	NC	2
472		min	0	3	-.002	3	0	15	4.626e-7	12	NC	1	8011.786	1
473		max	.002	1	0	2	.003	1	1.536e-5	1	NC	1	NC	2
474		min	0	3	-.001	3	0	15	4.626e-7	12	NC	1	9436.124	1
475		max	.001	1	0	2	.002	1	1.536e-5	1	NC	1	NC	1
476		min	0	3	-.001	3	0	15	4.626e-7	12	NC	1	NC	1
477		max	.001	1	0	2	.002	1	1.536e-5	1	NC	1	NC	1
478		min	0	3	-.001	3	0	15	4.626e-7	12	NC	1	NC	1
479		max	.001	1	0	2	.001	1	1.536e-5	1	NC	1	NC	1
480		min	0	3	-.001	3	0	15	4.626e-7	12	NC	1	NC	1
481		max	0	1	0	2	.001	1	1.536e-5	1	NC	1	NC	1
482		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
483		max	0	1	0	2	0	1	1.536e-5	1	NC	1	NC	1
484		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
485		max	0	1	0	2	0	1	1.536e-5	1	NC	1	NC	1
486		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
487		max	0	1	0	2	0	1	1.536e-5	1	NC	1	NC	1
488		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
489		max	0	1	0	2	0	1	1.536e-5	1	NC	1	NC	1
490		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
491		max	0	1	0	2	0	1	1.536e-5	1	NC	1	NC	1
492		min	0	3	0	3	0	15	4.626e-7	12	NC	1	NC	1
493		max	0	1	0	1	0	1	1.536e-5	1	NC	1	NC	1
494		min	0	1	0	1	0	1	4.626e-7	12	NC	1	NC	1
495	M1	max	.003	3	.11	1	0	1	1.911e-2	1	NC	1	NC	1
496		min	-.001	10	-.012	3	0	15	-1.77e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.003	3	.054	1	0	15	9.316e-3	1	NC	3	NC	1
498		min	-.001	10	-.006	3	-.005	1	-8.757e-3	3	2063.565	1	NC	1
499	3	max	.003	3	.005	3	0	15	1.722e-5	10	NC	5	NC	1
500		min	-.001	10	-.006	1	-.007	1	-1.379e-4	1	985.836	1	NC	1
501	4	max	.003	3	.024	3	0	15	5.178e-3	1	NC	5	NC	1
502		min	-.001	10	-.076	1	-.007	1	-3.153e-3	3	614.697	1	NC	1
503	5	max	.003	3	.049	3	0	15	1.049e-2	1	NC	15	NC	1
504		min	0	10	-.151	1	-.005	1	-6.221e-3	3	439.076	1	NC	1
505	6	max	.003	3	.076	3	0	15	1.581e-2	1	NC	15	NC	1
506		min	0	10	-.223	1	-.002	1	-9.288e-3	3	343.09	1	NC	1
507	7	max	.003	3	.102	3	0	1	2.112e-2	1	9413.062	15	NC	1
508		min	0	10	-.289	1	0	12	-1.236e-2	3	286.799	1	NC	1
509	8	max	.003	3	.124	3	0	1	2.644e-2	1	8359.901	15	NC	1
510		min	0	10	-.341	1	0	15	-1.542e-2	3	253.662	1	NC	1
511	9	max	.003	3	.139	3	0	15	2.901e-2	1	7811.107	15	NC	1
512		min	0	10	-.374	1	0	1	-1.548e-2	3	236.472	1	NC	1
513	10	max	.003	3	.144	3	0	1	2.977e-2	1	7644.054	15	NC	1
514		min	0	10	-.384	1	0	12	-1.352e-2	3	231.315	1	NC	1
515	11	max	.003	3	.141	3	0	1	3.052e-2	1	7810.967	15	NC	1
516		min	0	10	-.373	1	0	15	-1.157e-2	3	236.708	1	NC	1
517	12	max	.003	3	.129	3	0	15	2.873e-2	1	8359.607	15	NC	1
518		min	0	10	-.34	1	0	1	-9.636e-3	3	254.399	1	NC	1
519	13	max	.003	3	.109	3	0	15	2.308e-2	1	9412.545	15	NC	1
520		min	0	10	-.287	1	0	1	-7.713e-3	3	288.628	1	NC	1
521	14	max	.003	3	.085	3	.002	1	1.744e-2	1	NC	15	NC	1
522		min	0	10	-.221	1	0	15	-5.79e-3	3	347.039	1	NC	1
523	15	max	.003	3	.057	3	.004	1	1.18e-2	1	NC	15	NC	1
524		min	0	10	-.147	1	0	15	-3.868e-3	3	447.256	1	NC	1
525	16	max	.003	3	.029	3	.006	1	6.158e-3	1	NC	5	NC	1
526		min	0	10	-.073	1	0	15	-1.945e-3	3	632.017	1	NC	1
527	17	max	.002	3	.002	3	.007	1	5.153e-4	1	NC	5	NC	1
528		min	0	10	-.004	1	0	15	-2.259e-5	3	1025.213	1	NC	1
529	18	max	.002	3	.054	1	.005	1	1.087e-2	1	NC	5	NC	1
530		min	0	10	-.022	3	0	15	-3.135e-3	3	2163.897	1	NC	1
531	19	max	.002	3	.107	1	0	15	2.16e-2	1	NC	1	NC	1
532		min	0	10	-.044	3	0	1	-6.366e-3	3	NC	1	NC	1
533	M5	1	max	.01	.266	1	0	1	0	1	NC	1	NC	1
534		min	-.006	2	-.019	3	0	1	0	1	NC	1	NC	1
535	2	max	.01	3	.132	1	0	1	0	1	NC	5	NC	1
536		min	-.006	2	-.01	3	0	1	0	1	852.355	1	NC	1
537	3	max	.01	3	.015	3	0	1	0	1	NC	15	NC	1
538		min	-.006	2	-.021	1	0	1	0	1	398.198	1	NC	1
539	4	max	.01	3	.068	3	0	1	0	1	8874.354	15	NC	1
540		min	-.006	2	-.207	1	0	1	0	1	241.417	1	NC	1
541	5	max	.01	3	.141	3	0	1	0	1	6213.024	15	NC	1
542		min	-.006	2	-.412	1	0	1	0	1	168.638	1	NC	1
543	6	max	.01	3	.223	3	0	1	0	1	4784.93	15	NC	1
544		min	-.006	2	-.615	1	0	1	0	1	129.628	1	NC	1
545	7	max	.01	3	.302	3	0	1	0	1	3959.894	15	NC	1
546		min	-.006	2	-.801	1	0	1	0	1	107.11	1	NC	1
547	8	max	.009	3	.369	3	0	1	0	1	3480.041	15	NC	1
548		min	-.005	2	-.949	1	0	1	0	1	94.022	1	NC	1
549	9	max	.009	3	.412	3	0	1	0	1	3233.906	15	NC	1
550		min	-.005	2	-1.043	1	0	1	0	1	87.316	1	NC	1
551	10	max	.009	3	.427	3	0	1	0	1	3159.735	15	NC	1
552		min	-.005	2	-1.074	1	0	1	0	1	85.316	1	NC	1
553	11	max	.009	3	.417	3	0	1	0	1	3233.955	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.005	2	-1.042	1	0	1	0	1	87.41	1	NC	1
555		12	max	.009	3	.381	3	0	1	0	1	3480.159	15	NC	1
556			min	-.005	2	-.947	1	0	1	0	1	94.332	1	NC	1
557		13	max	.008	3	.322	3	0	1	0	1	3960.139	15	NC	1
558			min	-.005	2	-.795	1	0	1	0	1	107.917	1	NC	1
559		14	max	.008	3	.249	3	0	1	0	1	4785.415	15	NC	1
560			min	-.005	2	-.607	1	0	1	0	1	131.444	1	NC	1
561		15	max	.008	3	.167	3	0	1	0	1	6213.994	15	NC	1
562			min	-.005	2	-.401	1	0	1	0	1	172.571	1	NC	1
563		16	max	.008	3	.083	3	0	1	0	1	8876.399	15	NC	1
564			min	-.005	2	-.196	1	0	1	0	1	250.205	1	NC	1
565		17	max	.008	3	.006	3	0	1	0	1	NC	15	NC	1
566			min	-.005	2	-.013	1	0	1	0	1	419.5	1	NC	1
567		18	max	.008	3	.133	1	0	1	0	1	NC	5	NC	1
568			min	-.005	2	-.06	3	0	1	0	1	909.281	1	NC	1
569		19	max	.008	3	.258	1	0	1	0	1	NC	1	NC	1
570			min	-.005	2	-.12	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.003	3	.11	1	0	15	1.77e-2	3	NC	1	NC	1
572			min	-.001	10	-.012	3	0	1	-1.911e-2	1	NC	1	NC	1
573		2	max	.003	3	.054	1	.005	1	8.757e-3	3	NC	3	NC	1
574			min	-.001	10	-.006	3	0	15	-9.316e-3	1	2063.565	1	NC	1
575		3	max	.003	3	.005	3	.007	1	1.379e-4	1	NC	5	NC	1
576			min	-.001	10	-.006	1	0	15	-1.722e-5	10	985.836	1	NC	1
577		4	max	.003	3	.024	3	.007	1	3.153e-3	3	NC	5	NC	1
578			min	-.001	10	-.076	1	0	15	-5.178e-3	1	614.697	1	NC	1
579		5	max	.003	3	.049	3	.005	1	6.221e-3	3	NC	15	NC	1
580			min	0	10	-.151	1	0	15	-1.049e-2	1	439.076	1	NC	1
581		6	max	.003	3	.076	3	.002	1	9.288e-3	3	NC	15	NC	1
582			min	0	10	-.223	1	0	15	-1.581e-2	1	343.09	1	NC	1
583		7	max	.003	3	.102	3	0	12	1.236e-2	3	9413.062	15	NC	1
584			min	0	10	-.289	1	0	1	-2.112e-2	1	286.799	1	NC	1
585		8	max	.003	3	.124	3	0	15	1.542e-2	3	8359.901	15	NC	1
586			min	0	10	-.341	1	0	1	-2.644e-2	1	253.662	1	NC	1
587		9	max	.003	3	.139	3	0	1	1.548e-2	3	7811.107	15	NC	1
588			min	0	10	-.374	1	0	15	-2.901e-2	1	236.472	1	NC	1
589		10	max	.003	3	.144	3	0	12	1.352e-2	3	7644.054	15	NC	1
590			min	0	10	-.384	1	0	1	-2.977e-2	1	231.315	1	NC	1
591		11	max	.003	3	.141	3	0	15	1.157e-2	3	7810.967	15	NC	1
592			min	0	10	-.373	1	0	1	-3.052e-2	1	236.708	1	NC	1
593		12	max	.003	3	.129	3	0	1	9.636e-3	3	8359.607	15	NC	1
594			min	0	10	-.34	1	0	15	-2.873e-2	1	254.399	1	NC	1
595		13	max	.003	3	.109	3	0	1	7.713e-3	3	9412.545	15	NC	1
596			min	0	10	-.287	1	0	15	-2.308e-2	1	288.628	1	NC	1
597		14	max	.003	3	.085	3	0	15	5.79e-3	3	NC	15	NC	1
598			min	0	10	-.221	1	-.002	1	-1.744e-2	1	347.039	1	NC	1
599		15	max	.003	3	.057	3	0	15	3.868e-3	3	NC	15	NC	1
600			min	0	10	-.147	1	-.004	1	-1.18e-2	1	447.256	1	NC	1
601		16	max	.003	3	.029	3	0	15	1.945e-3	3	NC	5	NC	1
602			min	0	10	-.073	1	-.006	1	-6.158e-3	1	632.017	1	NC	1
603		17	max	.002	3	.002	3	0	15	2.259e-5	3	NC	5	NC	1
604			min	0	10	-.004	1	-.007	1	-5.153e-4	1	1025.213	1	NC	1
605		18	max	.002	3	.054	1	0	15	3.135e-3	3	NC	5	NC	1
606			min	0	10	-.022	3	-.005	1	-1.087e-2	1	2163.897	1	NC	1
607		19	max	.002	3	.107	1	0	1	6.366e-3	3	NC	1	NC	1
608			min	0	10	-.044	3	0	15	-2.16e-2	1	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

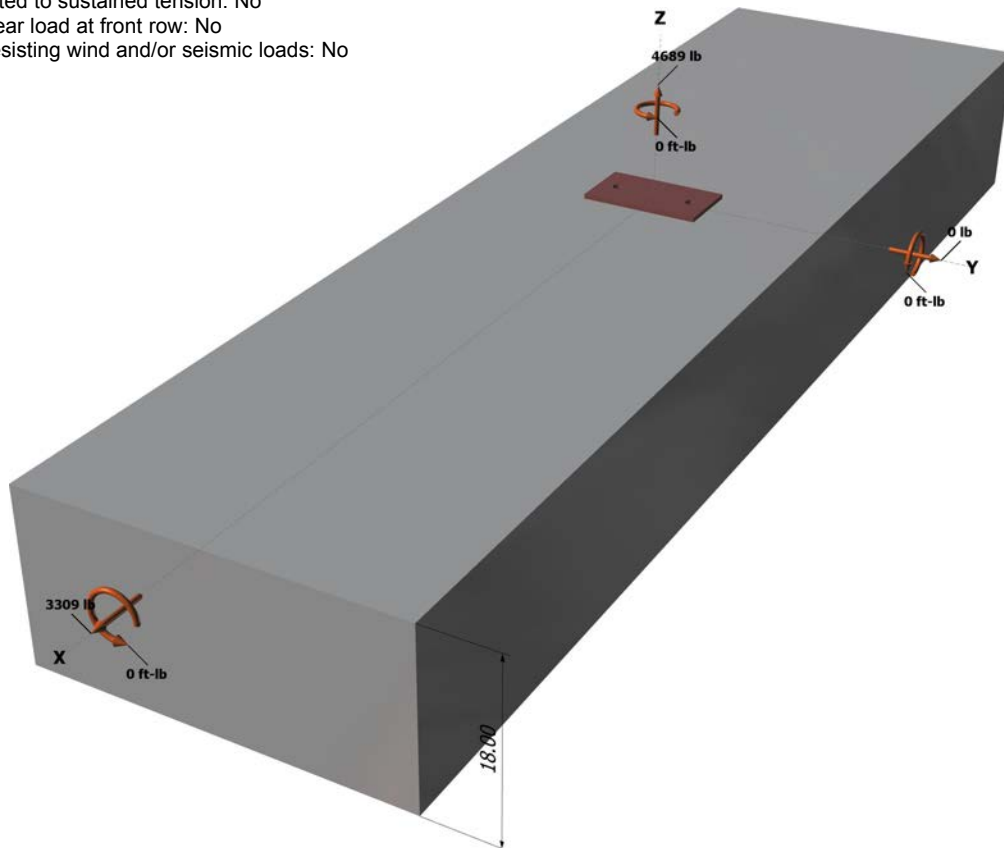
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

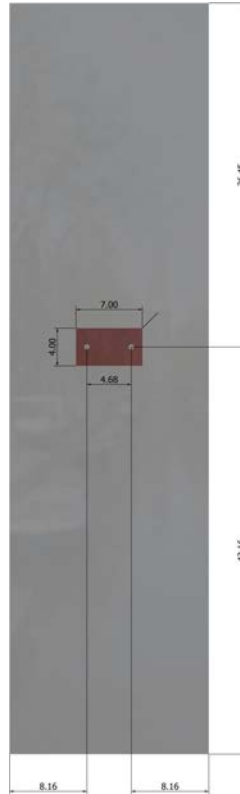
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpg} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.