

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	150 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 35.33 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

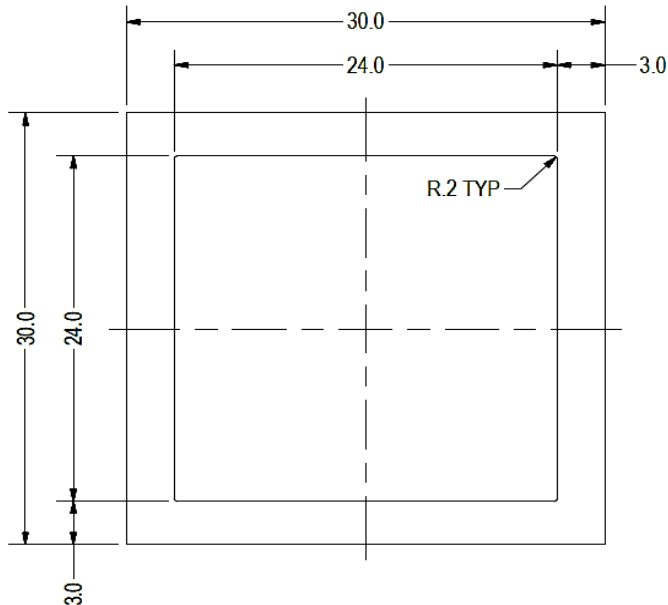
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.129 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.184 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	5%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.863 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	11%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.169 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

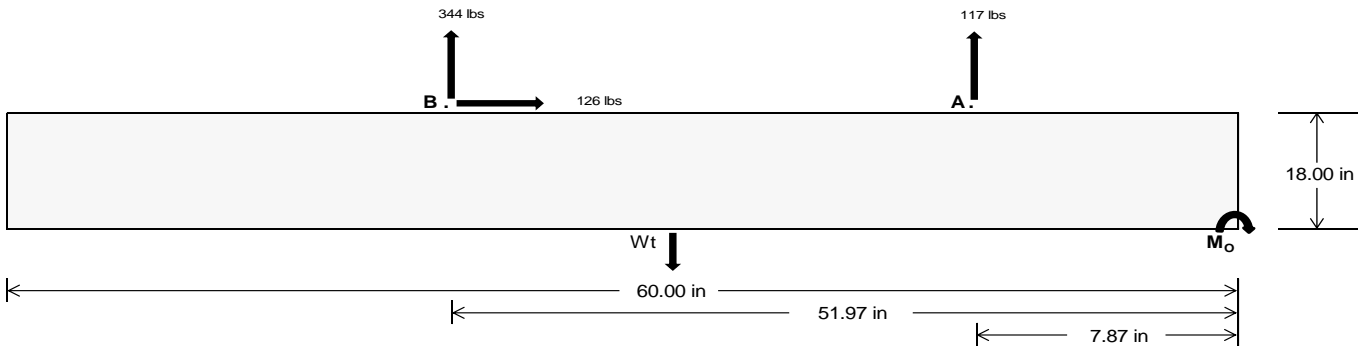
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	513.16	1495.36	k
Compressive Load =	1467.69	1057.89	k
Lateral Load =	24.22	547.40	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21083.0$ in-lbs
Resisting Force Required = 702.77 lbs
S.F. = 1.67
Weight Required = 1171.28 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 126.30 lbs
Friction = 0.4
Weight Required = 315.75 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 126.30 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	471 lbs	471 lbs	471 lbs	471 lbs	548 lbs	548 lbs	548 lbs	548 lbs	731 lbs	731 lbs	731 lbs	731 lbs	-234 lbs	-234 lbs	-234 lbs	-234 lbs
F_B	341 lbs	341 lbs	341 lbs	341 lbs	394 lbs	394 lbs	394 lbs	394 lbs	527 lbs	527 lbs	527 lbs	527 lbs	-688 lbs	-688 lbs	-688 lbs	-688 lbs
F_V	26 lbs	26 lbs	26 lbs	26 lbs	220 lbs	220 lbs	220 lbs	220 lbs	183 lbs	183 lbs	183 lbs	183 lbs	-253 lbs	-253 lbs	-253 lbs	-253 lbs
P_{total}	2716 lbs	2806 lbs	2897 lbs	2988 lbs	2845 lbs	2936 lbs	3026 lbs	3117 lbs	3161 lbs	3252 lbs	3343 lbs	3433 lbs	219 lbs	274 lbs	328 lbs	382 lbs
M	283 lbs-ft	283 lbs-ft	283 lbs-ft	283 lbs-ft	620 lbs-ft	620 lbs-ft	620 lbs-ft	620 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	449 lbs-ft	449 lbs-ft	449 lbs-ft	449 lbs-ft
e	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.22 ft	0.21 ft	0.20 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.20 ft	0.19 ft	2.05 ft	1.64 ft	1.37 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	271.5 psf	269.1 psf	266.8 psf	264.8 psf	240.1 psf	239.1 psf	238.2 psf	237.3 psf	270.9 psf	268.4 psf	266.2 psf	264.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	349.2 psf	343.2 psf	337.7 psf	332.7 psf	410.1 psf	401.4 psf	393.4 psf	386.0 psf	451.7 psf	441.1 psf	431.4 psf	422.4 psf	185.5 psf	116.0 psf	101.0 psf	96.2 psf

Maximum Bearing Pressure = 452 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

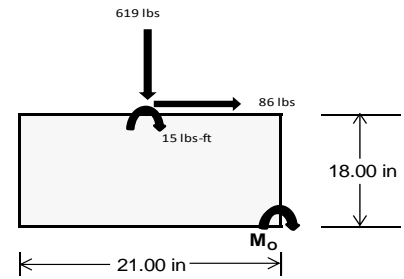
Overturning Check

$M_o = 397.9$ ft-lbs
 Resisting Force Required = 454.74 lbs
 S.F. = 1.67
 Weight Required = 757.90 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	105 lbs	90 lbs	55 lbs	256 lbs	619 lbs	217 lbs	66 lbs	-12 lbs	18 lbs
F_v	14 lbs	114 lbs	14 lbs	10 lbs	86 lbs	10 lbs	14 lbs	114 lbs	14 lbs
P_{total}	2461 lbs	2446 lbs	2411 lbs	2498 lbs	2862 lbs	2460 lbs	755 lbs	677 lbs	707 lbs
M	38 lbs-ft	191 lbs-ft	39 lbs-ft	28 lbs-ft	144 lbs-ft	31 lbs-ft	38 lbs-ft	191 lbs-ft	39 lbs-ft
e	0.02 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.28 ft	0.06 ft
$L/6$	0.29 ft	1.59 ft	1.72 ft	1.73 ft	1.65 ft	1.73 ft	1.65 ft	1.19 ft	1.64 ft
f_{min}	266.2 sqft	204.7 sqft	260.1 sqft	274.7 sqft	270.8 sqft	269.2 sqft	71.2 sqft	2.7 sqft	65.4 sqft
f_{max}	296.3 psf	354.3 psf	291.0 psf	296.3 psf	383.3 psf	293.1 psf	101.4 psf	152.1 psf	96.3 psf



Maximum Bearing Pressure = 383 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

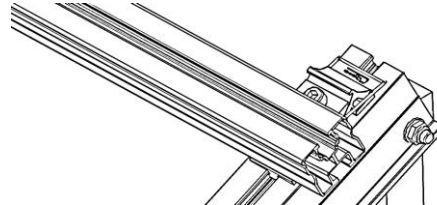
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.677 k
Allowable Uplift =	1.214 k
Utilization =	<u>56%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.083 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.129 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Diagonal Strut

Maximum Axial Load =	0.184 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.138 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.169 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.061 in
	<u>0.061 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

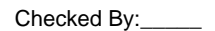
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 15° 150mph 30psf 4.5ft 7-10.rdb Page 20



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 15° 150mph 30psf 4.5ft 7-10.r



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	270.889	1	.132	6	.2	1	0	10	0	4	0	15
30			min	-356.623	3	.029	15	-.401	5	0	4	0	3	0	6
31		16	max	270.985	1	.094	6	.2	1	0	10	0	4	0	15
32			min	-356.55	3	.02	15	-.488	5	0	4	0	3	0	6
33		17	max	271.082	1	.064	2	.2	1	0	10	0	1	0	15
34			min	-356.478	3	.011	15	-.575	5	0	4	0	3	0	6
35		18	max	271.178	1	.034	2	.2	1	0	10	0	1	0	15
36			min	-356.406	3	0	9	-.663	5	0	4	0	3	0	6
37		19	max	271.274	1	.007	10	.2	1	0	10	0	1	0	15
38			min	-356.333	3	-.027	1	-.75	5	0	4	0	3	0	6
39	M3	1	max	48.764	2	1.812	6	-.003	10	0	5	0	4	0	6
40			min	-55.49	9	.425	15	-1.357	4	0	1	0	10	0	15
41		2	max	48.697	2	1.634	6	-.003	10	0	5	0	1	0	6
42			min	-55.546	9	.383	15	-1.224	4	0	1	0	10	0	15
43		3	max	48.63	2	1.456	6	-.003	10	0	5	0	1	0	2
44			min	-55.602	9	.341	15	-1.09	4	0	1	0	10	0	15
45		4	max	48.563	2	1.278	6	-.003	10	0	5	0	1	0	15
46			min	-55.658	9	.299	15	-.957	4	0	1	0	5	0	4
47		5	max	48.496	2	1.1	6	-.003	10	0	5	0	1	0	15
48			min	-55.714	9	.257	15	-.823	4	0	1	0	5	0	4
49		6	max	48.429	2	.922	6	-.003	10	0	5	0	1	0	15
50			min	-55.77	9	.215	15	-.69	4	0	1	0	5	0	4
51		7	max	48.362	2	.744	6	-.003	10	0	5	0	1	0	15
52			min	-55.826	9	.174	15	-.556	4	0	1	0	5	0	4
53		8	max	48.294	2	.566	6	-.003	10	0	5	0	1	0	15
54			min	-55.882	9	.132	15	-.422	4	0	1	0	5	0	4
55		9	max	48.227	2	.388	6	-.003	10	0	5	0	1	0	15
56			min	-55.938	9	.09	15	-.289	4	0	1	0	5	-.001	4
57		10	max	48.16	2	.21	6	-.003	10	0	5	0	1	0	15
58			min	-55.994	9	.048	15	-.186	1	0	1	0	5	-.001	4
59		11	max	48.093	2	.038	2	.017	5	0	5	0	1	0	15
60			min	-56.049	9	.006	15	-.186	1	0	1	0	5	-.001	4
61		12	max	48.026	2	-.036	15	.15	5	0	5	0	1	0	15
62			min	-56.105	9	-.146	4	-.186	1	0	1	0	5	-.001	4
63		13	max	47.959	2	-.078	15	.284	5	0	5	0	1	0	15
64			min	-56.161	9	-.324	4	-.186	1	0	1	0	5	-.001	4
65		14	max	47.892	2	-.119	15	.417	5	0	5	0	9	0	15
66			min	-56.217	9	-.502	4	-.186	1	0	1	0	5	-.001	4
67		15	max	47.825	2	-.161	15	.551	5	0	5	0	10	0	15
68			min	-56.273	9	-.68	4	-.186	1	0	1	0	4	0	4
69		16	max	47.758	2	-.203	15	.684	5	0	5	0	10	0	15
70			min	-56.329	9	-.858	4	-.186	1	0	1	0	4	0	4
71		17	max	47.691	2	-.245	15	.818	5	0	5	0	10	0	15
72			min	-56.385	9	-1.037	4	-.186	1	0	1	0	4	0	4
73		18	max	47.623	2	-.287	15	.951	5	0	5	0	10	0	15
74			min	-56.441	9	-1.215	4	-.186	1	0	1	0	1	0	4
75		19	max	47.556	2	-.329	15	1.085	5	0	5	0	5	0	1
76			min	-56.497	9	-1.393	4	-.186	1	0	1	0	1	0	1
77	M4	1	max	373.102	1	0	1	-.001	10	0	1	0	5	0	1
78			min	-116.166	3	0	1	-17.434	4	0	1	0	2	0	1
79		2	max	373.167	1	0	1	-.001	10	0	1	0	12	0	1
80			min	-116.117	3	0	1	-17.491	4	0	1	-.002	4	0	1
81		3	max	373.231	1	0	1	-.001	10	0	1	0	10	0	1
82			min	-116.068	3	0	1	-17.547	4	0	1	-.003	4	0	1
83		4	max	373.296	1	0	1	-.001	10	0	1	0	10	0	1
84			min	-116.02	3	0	1	-17.603	4	0	1	-.005	4	0	1
85		5	max	373.361	1	0	1	-.001	10	0	1	0	10	0	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-115.971	3	0	1	-17.659	4	0	1	-.006	4	0	1
87	6	max	373.426	1	0	1	-.001	10	0	1	0	10	0	1
88		min	-115.923	3	0	1	-17.715	4	0	1	-.008	4	0	1
89	7	max	373.49	1	0	1	-.001	10	0	1	0	10	0	1
90		min	-115.874	3	0	1	-17.771	4	0	1	-.009	4	0	1
91	8	max	373.555	1	0	1	-.001	10	0	1	0	10	0	1
92		min	-115.826	3	0	1	-17.827	4	0	1	-.011	4	0	1
93	9	max	373.62	1	0	1	-.001	10	0	1	0	10	0	1
94		min	-115.777	3	0	1	-17.883	4	0	1	-.013	4	0	1
95	10	max	373.684	1	0	1	-.001	10	0	1	0	10	0	1
96		min	-115.729	3	0	1	-17.939	4	0	1	-.014	4	0	1
97	11	max	373.749	1	0	1	-.001	10	0	1	0	10	0	1
98		min	-115.68	3	0	1	-17.995	4	0	1	-.016	4	0	1
99	12	max	373.814	1	0	1	-.001	10	0	1	0	10	0	1
100		min	-115.632	3	0	1	-18.051	4	0	1	-.017	4	0	1
101	13	max	373.879	1	0	1	-.001	10	0	1	0	10	0	1
102		min	-115.583	3	0	1	-18.107	4	0	1	-.019	4	0	1
103	14	max	373.943	1	0	1	-.001	10	0	1	0	10	0	1
104		min	-115.535	3	0	1	-18.163	4	0	1	-.021	4	0	1
105	15	max	374.008	1	0	1	-.001	10	0	1	0	10	0	1
106		min	-115.486	3	0	1	-18.22	4	0	1	-.022	4	0	1
107	16	max	374.073	1	0	1	-.001	10	0	1	0	10	0	1
108		min	-115.438	3	0	1	-18.276	4	0	1	-.024	4	0	1
109	17	max	374.137	1	0	1	-.001	10	0	1	0	10	0	1
110		min	-115.389	3	0	1	-18.332	4	0	1	-.026	4	0	1
111	18	max	374.202	1	0	1	-.001	10	0	1	0	10	0	1
112		min	-115.341	3	0	1	-18.388	4	0	1	-.027	4	0	1
113	19	max	374.267	1	0	1	-.001	10	0	1	0	10	0	1
114		min	-115.292	3	0	1	-18.444	4	0	1	-.029	4	0	1
115	M6	1	max	861.423	1	.648	.866	4	0	3	0	3	0	1
116		min	-1138.46	3	.15	15	-.25	3	0	5	0	1	0	1
117	2	max	861.519	1	.61	6	.779	4	0	3	0	4	0	15
118		min	-1138.388	3	.141	15	-.25	3	0	5	0	2	0	6
119	3	max	861.616	1	.572	6	.692	4	0	3	0	4	0	15
120		min	-1138.316	3	.132	15	-.25	3	0	5	0	2	0	6
121	4	max	861.712	1	.534	6	.604	4	0	3	0	4	0	15
122		min	-1138.243	3	.123	15	-.25	3	0	5	0	2	0	6
123	5	max	861.809	1	.496	6	.517	4	0	3	0	4	0	15
124		min	-1138.171	3	.115	15	-.25	3	0	5	0	3	0	6
125	6	max	861.905	1	.459	6	.43	4	0	3	0	4	0	15
126		min	-1138.099	3	.106	15	-.25	3	0	5	0	3	0	6
127	7	max	862.001	1	.421	6	.342	4	0	3	0	4	0	15
128		min	-1138.027	3	.097	15	-.25	3	0	5	0	3	0	6
129	8	max	862.098	1	.383	6	.255	4	0	3	0	4	0	15
130		min	-1137.954	3	.088	15	-.25	3	0	5	0	3	0	6
131	9	max	862.194	1	.345	6	.168	4	0	3	0	4	0	15
132		min	-1137.882	3	.079	15	-.25	3	0	5	0	3	0	6
133	10	max	862.29	1	.307	6	.08	4	0	3	0	4	0	15
134		min	-1137.81	3	.07	15	-.25	3	0	5	0	3	0	6
135	11	max	862.387	1	.269	6	.055	9	0	3	0	4	0	15
136		min	-1137.737	3	.061	15	-.25	3	0	5	0	3	0	6
137	12	max	862.483	1	.239	2	.055	9	0	3	0	4	0	15
138		min	-1137.665	3	.052	15	-.25	3	0	5	0	3	0	6
139	13	max	862.579	1	.21	2	.055	9	0	3	0	4	0	15
140		min	-1137.593	3	.043	15	-.25	3	0	5	0	3	0	6
141	14	max	862.676	1	.18	2	.055	9	0	3	0	4	0	15
142		min	-1137.521	3	.035	15	-.29	5	0	5	0	3	0	6



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	862.772	1	.151	2	.055	9	0	3	0	4	0	15
144		min	-1137.448	3	.026	15	-.377	5	0	5	0	3	0	6
145	16	max	862.869	1	.121	2	.055	9	0	3	0	4	0	15
146		min	-1137.376	3	.017	15	-.464	5	0	5	0	3	0	6
147	17	max	862.965	1	.092	2	.055	9	0	3	0	4	0	15
148		min	-1137.304	3	-.003	9	-.552	5	0	5	0	3	0	6
149	18	max	863.061	1	.062	2	.055	9	0	3	0	4	0	15
150		min	-1137.232	3	-.028	9	-.639	5	0	5	0	3	0	6
151	19	max	863.158	1	.033	2	.055	9	0	3	0	4	0	15
152		min	-1137.159	3	-.052	9	-.726	5	0	5	0	3	0	6
153	M7	1	max	184.006	2	1.817	4	0	2	0	1	0	4	4
154		min	-100.03	9	.431	15	-1.42	4	0	3	0	3	0	15
155	2	max	183.939	2	1.639	4	0	2	0	1	0	4	0	4
156		min	-100.085	9	.389	15	-1.287	4	0	3	0	3	0	15
157	3	max	183.872	2	1.461	4	0	2	0	1	0	4	0	2
158		min	-100.141	9	.347	15	-1.153	4	0	3	0	3	0	9
159	4	max	183.805	2	1.283	4	0	2	0	1	0	1	0	15
160		min	-100.197	9	.305	15	-1.02	4	0	3	0	3	0	9
161	5	max	183.738	2	1.105	4	0	2	0	1	0	1	0	15
162		min	-100.253	9	.264	15	-.886	4	0	3	0	5	0	6
163	6	max	183.67	2	.927	4	0	2	0	1	0	1	0	15
164		min	-100.309	9	.222	15	-.753	4	0	3	0	5	0	6
165	7	max	183.603	2	.749	4	0	2	0	1	0	1	0	15
166		min	-100.365	9	.18	15	-.619	4	0	3	0	5	0	6
167	8	max	183.536	2	.571	4	0	2	0	1	0	1	0	15
168		min	-100.421	9	.138	15	-.486	4	0	3	0	5	0	6
169	9	max	183.469	2	.393	4	0	2	0	1	0	1	0	15
170		min	-100.477	9	.096	15	-.352	4	0	3	0	5	-.001	6
171	10	max	183.402	2	.215	4	0	2	0	1	0	1	0	15
172		min	-100.533	9	.054	15	-.219	4	0	3	0	5	-.001	6
173	11	max	183.335	2	.056	2	0	2	0	1	0	1	0	15
174		min	-100.589	9	-.001	9	-.085	4	0	3	0	5	-.001	6
175	12	max	183.268	2	-.029	15	.051	5	0	1	0	1	0	15
176		min	-100.645	9	-.141	6	-.012	1	0	3	0	5	-.001	6
177	13	max	183.201	2	-.071	15	.184	5	0	1	0	1	0	15
178		min	-100.7	9	-.319	6	-.012	1	0	3	0	5	-.001	6
179	14	max	183.134	2	-.113	15	.318	5	0	1	0	1	0	15
180		min	-100.756	9	-.497	6	-.012	1	0	3	0	5	-.001	6
181	15	max	183.067	2	-.155	15	.451	5	0	1	0	1	0	15
182		min	-100.812	9	-.675	6	-.012	1	0	3	0	5	0	6
183	16	max	182.999	2	-.197	15	.585	5	0	1	0	1	0	15
184		min	-100.868	9	-.853	6	-.012	1	0	3	0	5	0	6
185	17	max	182.932	2	-.239	15	.718	5	0	1	0	1	0	15
186		min	-100.924	9	-1.031	6	-.012	1	0	3	0	5	0	6
187	18	max	182.865	2	-.28	15	.852	5	0	1	0	1	0	15
188		min	-100.98	9	-1.209	6	-.012	1	0	3	0	5	0	6
189	19	max	182.798	2	-.322	15	.985	5	0	1	0	1	0	1
190		min	-101.036	9	-1.387	6	-.012	1	0	3	0	3	0	1
191	M8	1	max	1127.831	1	0	.163	1	0	1	0	4	0	1
192		min	-395.61	3	0	1	-17.81	4	0	1	0	1	0	1
193	2	max	1127.896	1	0	1	.163	1	0	1	0	1	0	1
194		min	-395.562	3	0	1	-17.866	4	0	1	-.002	4	0	1
195	3	max	1127.961	1	0	1	.163	1	0	1	0	1	0	1
196		min	-395.513	3	0	1	-17.922	4	0	1	-.003	4	0	1
197	4	max	1128.025	1	0	1	.163	1	0	1	0	1	0	1
198		min	-395.464	3	0	1	-17.978	4	0	1	-.005	4	0	1
199	5	max	1128.09	1	0	1	.163	1	0	1	0	1	0	1





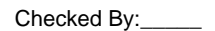
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	272.525	1	.161	4	-.005	10	0	1	0	5	0	15
258		min	-327.666	3	.049	15	-.22	4	-.001	5	0	3	0	4
259	16	max	272.621	1	.123	4	-.005	10	0	1	0	5	0	15
260		min	-327.593	3	.04	15	-.308	4	-.001	5	0	3	0	4
261	17	max	272.718	1	.085	4	-.005	10	0	1	0	5	0	15
262		min	-327.521	3	.024	9	-.395	4	-.001	5	0	3	0	4
263	18	max	272.814	1	.055	3	-.005	10	0	1	0	5	0	15
264		min	-327.449	3	-.001	9	-.482	4	-.001	5	0	3	0	4
265	19	max	272.91	1	.033	3	-.005	10	0	1	0	5	0	15
266		min	-327.376	3	-.028	1	-.57	4	-.001	5	0	3	0	4
267	M11	1	max	48.321	2	1.811	.203	1	0	4	.001	5	0	6
268		min	-55.573	9	.424	15	-1.222	5	0	10	0	1	0	15
269	2	max	48.254	2	1.633	6	.203	1	0	4	0	5	0	6
270		min	-55.629	9	.382	15	-1.089	5	0	10	0	1	0	15
271	3	max	48.187	2	1.455	6	.203	1	0	4	0	5	0	2
272		min	-55.685	9	.34	15	-.955	5	0	10	0	1	0	3
273	4	max	48.12	2	1.277	6	.203	1	0	4	0	5	0	15
274		min	-55.741	9	.298	15	-.822	5	0	10	0	1	0	4
275	5	max	48.052	2	1.099	6	.203	1	0	4	0	3	0	15
276		min	-55.797	9	.257	15	-.688	5	0	10	0	1	0	4
277	6	max	47.985	2	.921	6	.203	1	0	4	0	3	0	15
278		min	-55.853	9	.215	15	-.554	5	0	10	0	1	0	4
279	7	max	47.918	2	.743	6	.203	1	0	4	0	3	0	15
280		min	-55.909	9	.173	15	-.421	5	0	10	0	1	0	4
281	8	max	47.851	2	.565	6	.203	1	0	4	0	3	0	15
282		min	-55.965	9	.131	15	-.287	5	0	10	0	1	0	4
283	9	max	47.784	2	.387	6	.203	1	0	4	0	3	0	15
284		min	-56.021	9	.089	15	-.154	5	0	10	0	1	-.001	4
285	10	max	47.717	2	.209	6	.203	1	0	4	0	3	0	15
286		min	-56.077	9	.047	15	-.02	5	0	10	0	4	-.001	4
287	11	max	47.65	2	.038	2	.203	1	0	4	0	3	0	15
288		min	-56.133	9	.002	3	-.017	3	0	10	0	4	-.001	4
289	12	max	47.583	2	-.036	15	.289	4	0	4	0	3	0	15
290		min	-56.188	9	-.147	4	-.017	3	0	10	0	4	-.001	4
291	13	max	47.516	2	-.078	15	.422	4	0	4	0	3	0	15
292		min	-56.244	9	-.325	4	-.017	3	0	10	0	5	-.001	4
293	14	max	47.449	2	-.12	15	.556	4	0	4	0	3	0	15
294		min	-56.3	9	-.503	4	-.017	3	0	10	0	10	-.001	4
295	15	max	47.381	2	-.162	15	.69	4	0	4	0	3	0	15
296		min	-56.356	9	-.681	4	-.017	3	0	10	0	10	0	4
297	16	max	47.314	2	-.204	15	.823	4	0	4	0	4	0	15
298		min	-56.412	9	-.859	4	-.017	3	0	10	0	10	0	4
299	17	max	47.247	2	-.246	15	.957	4	0	4	0	4	0	15
300		min	-56.468	9	-1.037	4	-.017	3	0	10	0	10	0	4
301	18	max	47.18	2	-.287	15	1.09	4	0	4	0	4	0	15
302		min	-56.524	9	-1.215	4	-.017	3	0	10	0	10	0	4
303	19	max	47.113	2	-.329	15	1.224	4	0	4	.001	4	0	1
304		min	-56.58	9	-1.393	4	-.017	3	0	10	0	10	0	1
305	M12	1	max	373.229	1	0	.781	1	0	1	0	4	0	1
306		min	-115.809	3	0	1	-16.331	5	0	1	0	3	0	1
307	2	max	373.293	1	0	1	.781	1	0	1	0	1	0	1
308		min	-115.761	3	0	1	-16.388	5	0	1	-.001	5	0	1
309	3	max	373.358	1	0	1	.781	1	0	1	0	1	0	1
310		min	-115.712	3	0	1	-16.444	5	0	1	-.003	5	0	1
311	4	max	373.423	1	0	1	.781	1	0	1	0	1	0	1
312		min	-115.664	3	0	1	-16.5	5	0	1	-.004	5	0	1
313	5	max	373.488	1	0	1	.781	1	0	1	0	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	70.667	1	1.721	9	-.257	10	0	5	0	10	.147	2
372			min	-5.772	3	-23.755	3	-16.881	1	0	1	-.018	1	-.086	3
373		16	max	67.419	2	13.888	10	-.26	10	0	1	0	10	.15	2
374			min	-33.81	3	-49.203	3	-17.053	1	0	4	-.022	1	-.081	3
375		17	max	67.491	2	13.663	10	-.26	10	0	1	0	10	.148	2
376			min	-33.756	3	-49.405	3	-17.053	1	0	4	-.026	1	-.07	3
377		18	max	-2.382	12	342.464	2	-.26	10	0	3	0	10	.075	2
378			min	-59.771	1	-161.891	3	-27.44	4	0	2	-.03	1	-.035	3
379		19	max	-2.346	12	342.194	2	-.26	10	0	3	0	10	0	2
380			min	-59.699	1	-162.093	3	-27.198	4	0	2	-.033	1	0	3
381	M5	1	max	142.752	1	1092.563	3	0	10	0	1	.032	4	0	3
382			min	.212	15	-879.73	1	-45.512	3	0	5	0	10	0	1
383		2	max	142.825	1	1092.361	3	0	10	0	1	.028	4	.19	1
384			min	.234	15	-880	1	-45.512	3	0	5	-.004	3	-.236	3
385		3	max	173.526	1	6.727	9	4.988	3	0	3	.023	4	.378	1
386			min	-39.759	3	-74.962	3	-17.655	4	0	4	-.013	3	-.468	3
387		4	max	173.598	1	6.502	9	4.988	3	0	3	.019	4	.382	1
388			min	-39.704	3	-75.164	3	-17.413	4	0	4	-.012	3	-.452	3
389		5	max	173.671	1	6.278	9	4.988	3	0	3	.015	4	.386	1
390			min	-39.65	3	-75.367	3	-17.171	4	0	4	-.011	3	-.436	3
391		6	max	173.743	1	6.053	9	4.988	3	0	3	.012	4	.391	1
392			min	-39.596	3	-75.569	3	-16.929	4	0	4	-.01	3	-.419	3
393		7	max	173.815	1	5.828	9	4.988	3	0	3	.008	4	.395	1
394			min	-39.542	3	-75.771	3	-16.687	4	0	4	-.009	3	-.403	3
395		8	max	173.887	1	5.603	9	4.988	3	0	3	.004	4	.399	1
396			min	-39.488	3	-75.974	3	-16.445	4	0	4	-.008	3	-.386	3
397		9	max	173.96	1	5.378	9	4.988	3	0	3	0	4	.405	2
398			min	-39.433	3	-76.176	3	-16.203	4	0	4	-.006	3	-.37	3
399		10	max	174.032	1	5.154	9	4.988	3	0	3	0	2	.417	2
400			min	-39.379	3	-76.378	3	-15.961	4	0	4	-.005	3	-.353	3
401		11	max	174.104	1	4.929	9	4.988	3	0	3	0	2	.429	2
402			min	-39.325	3	-76.58	3	-15.719	4	0	4	-.006	4	-.337	3
403		12	max	174.177	1	4.704	9	4.988	3	0	3	0	2	.441	2
404			min	-39.271	3	-76.783	3	-15.477	4	0	4	-.009	4	-.32	3
405		13	max	174.249	1	4.479	9	4.988	3	0	3	0	2	.453	2
406			min	-39.217	3	-76.985	3	-15.235	4	0	4	-.013	4	-.303	3
407		14	max	174.321	1	4.255	9	4.988	3	0	3	0	2	.466	2
408			min	-39.162	3	-77.187	3	-14.993	4	0	4	-.016	4	-.287	3
409		15	max	174.393	1	4.03	9	4.988	3	0	3	0	3	.478	2
410			min	-39.108	3	-77.39	3	-14.751	4	0	4	-.019	4	-.27	3
411		16	max	223.679	2	64.026	2	4.963	3	0	3	0	3	.489	2
412			min	-105.977	3	-135.616	3	-13.53	4	0	4	-.023	4	-.253	3
413		17	max	223.751	2	63.756	2	4.963	3	0	3	.002	3	.475	2
414			min	-105.923	3	-135.818	3	-13.288	4	0	4	-.026	4	-.223	3
415		18	max	-2.678	12	1107.074	2	4.574	3	0	4	.003	3	.239	2
416			min	-142.916	1	-518.969	3	-30.448	5	0	1	-.032	4	-.112	3
417		19	max	-2.642	12	1106.804	2	4.574	3	0	4	.004	3	0	3
418			min	-142.844	1	-519.171	3	-30.206	5	0	1	-.039	4	0	2
419	M9	1	max	59.627	1	336.485	3	124.86	4	0	3	0	5	0	1
420			min	-.24	5	-271.899	1	.262	10	0	1	-.033	1	0	3
421		2	max	59.699	1	336.283	3	125.102	4	0	3	.027	5	.059	1
422			min	-.212	15	-272.168	1	.262	10	0	1	-.029	1	-.073	3
423		3	max	70.054	1	4.404	9	16.592	1	0	4	.052	5	.117	1
424			min	-6.529	3	-21.25	3	-23.384	5	0	10	-.025	1	-.145	3
425		4	max	70.126	1	4.179	9	16.592	1	0	4	.047	5	.118	1
426			min	-6.474	3	-21.452	3	-23.142	5	0	10	-.022	1	-.14	3
427		5	max	70.199	1	3.954	9	16.592	1	0	4	.042	5	.119	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-6.42	3	-21.655	3	-22.9	5	0	10	-.018	1	-.135	3
429	6	max	70.271	1	3.729	9	16.592	1	0	4	.037	5	.119	1
430		min	-6.366	3	-21.857	3	-22.658	5	0	10	-.014	1	-.131	3
431	7	max	70.343	1	3.505	9	16.592	1	0	4	.032	5	.12	1
432		min	-6.312	3	-22.059	3	-22.416	5	0	10	-.011	1	-.126	3
433	8	max	70.415	1	3.28	9	16.592	1	0	4	.027	5	.121	1
434		min	-6.258	3	-22.262	3	-22.174	5	0	10	-.007	1	-.121	3
435	9	max	70.488	1	3.055	9	16.592	1	0	4	.022	5	.124	2
436		min	-6.203	3	-22.464	3	-21.932	5	0	10	-.004	1	-.116	3
437	10	max	70.56	1	2.83	9	16.592	1	0	4	.018	4	.128	2
438		min	-6.149	3	-22.666	3	-21.69	5	0	10	0	1	-.111	3
439	11	max	70.632	1	2.606	9	16.592	1	0	4	.014	4	.131	2
440		min	-6.095	3	-22.868	3	-21.448	5	0	10	0	10	-.106	3
441	12	max	70.705	1	2.381	9	16.592	1	0	4	.01	4	.135	2
442		min	-6.041	3	-23.071	3	-21.206	5	0	10	0	10	-.101	3
443	13	max	70.777	1	2.156	9	16.592	1	0	4	.011	1	.139	2
444		min	-5.986	3	-23.273	3	-20.964	5	0	10	0	10	-.096	3
445	14	max	70.849	1	1.931	9	16.592	1	0	4	.014	1	.143	2
446		min	-5.932	3	-23.475	3	-20.722	5	0	10	0	5	-.091	3
447	15	max	70.921	1	1.706	9	16.592	1	0	4	.018	1	.146	2
448		min	-5.878	3	-23.678	3	-20.48	5	0	10	-.005	5	-.086	3
449	16	max	67.49	2	13.665	10	16.783	1	0	10	.022	1	.15	2
450		min	-34.372	3	-49.543	3	-19.059	5	0	4	-.008	5	-.081	3
451	17	max	67.563	2	13.44	10	16.783	1	0	10	.025	1	.148	2
452		min	-34.318	3	-49.745	3	-18.817	5	0	4	-.012	5	-.07	3
453	18	max	8.331	5	342.464	2	17.547	1	0	2	.029	1	.075	2
454		min	-59.649	1	-161.886	3	-34.892	5	0	3	-.02	5	-.035	3
455	19	max	8.364	5	342.194	2	17.547	1	0	2	.033	1	0	2
456		min	-59.577	1	-162.088	3	-34.65	5	0	3	-.028	5	0	3
457	M13	1	max	124.859	4	271.714	1	.24	5	0	.033	1	0	1
458		min	.262	10	-336.507	3	-59.624	1	0	3	0	5	0	3
459	2	max	119.948	4	192.738	1	.924	5	0	1	.008	3	.144	3
460		min	.262	10	-238.423	3	-45.061	1	0	3	-.001	10	-.116	1
461	3	max	115.038	4	113.761	1	1.608	5	0	1	.006	3	.238	3
462		min	.262	10	-140.338	3	-30.499	1	0	3	-.012	1	-.193	1
463	4	max	110.127	4	34.785	1	2.293	5	0	1	.004	3	.284	3
464		min	.262	10	-42.253	3	-15.937	1	0	3	-.024	1	-.23	1
465	5	max	105.216	4	55.832	3	2.977	5	0	1	.002	3	.281	3
466		min	.262	10	-44.191	1	-2.598	3	0	3	-.028	1	-.228	1
467	6	max	100.305	4	153.917	3	13.188	1	0	1	.004	5	.228	3
468		min	.262	10	-123.168	1	-1.954	3	0	3	-.025	1	-.186	1
469	7	max	95.394	4	252.001	3	27.75	1	0	1	.006	5	.127	3
470		min	.262	10	-202.144	1	-1.311	3	0	3	-.015	1	-.104	1
471	8	max	90.483	4	350.086	3	42.312	1	0	1	.009	4	.016	1
472		min	.262	10	-281.12	1	-.667	3	0	3	0	12	-.024	3
473	9	max	85.573	4	448.171	3	56.874	1	0	1	.028	1	.177	1
474		min	.262	10	-360.096	1	-.023	3	0	3	0	3	-.223	3
475	10	max	80.662	4	546.256	3	71.436	1	0	1	.06	1	.377	1
476		min	.262	10	-439.073	1	.58	12	0	3	-.018	5	-.472	3
477	11	max	55.54	4	360.096	1	6.508	5	0	3	.027	1	.177	1
478		min	.261	10	-448.171	3	-56.76	1	0	1	-.014	5	-.223	3
479	12	max	50.63	4	281.12	1	7.192	5	0	3	.003	2	.016	1
480		min	.261	10	-350.086	3	-42.198	1	0	1	-.011	5	-.024	3
481	13	max	45.719	4	202.144	1	7.876	5	0	3	0	10	.127	3
482		min	.261	10	-252.001	3	-27.636	1	0	1	-.015	1	-.104	1
483	14	max	40.808	4	123.168	1	8.561	5	0	3	-.002	10	.228	3
484		min	.261	10	-153.917	3	-13.074	1	0	1	-.025	1	-.186	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	35.897	4	44.191	1	9.805	4	0	3	.001	5	.281	3
486		min	.261	10	-55.832	3	-1.043	2	0	1	-.028	1	-.228	1
487	16	max	30.986	4	42.253	3	16.051	1	0	3	.006	5	.284	3
488		min	.261	10	-34.785	1	.251	10	0	1	-.023	1	-.23	1
489	17	max	26.075	4	140.338	3	30.613	1	0	3	.011	5	.238	3
490		min	.261	10	-113.761	1	1.473	10	0	1	-.012	1	-.193	1
491	18	max	21.165	4	238.423	3	45.175	1	0	3	.019	4	.144	3
492		min	.261	10	-192.738	1	2.695	10	0	1	-.001	10	-.116	1
493	19	max	17.045	1	336.507	3	59.737	1	0	3	.033	1	0	1
494		min	.261	10	-271.714	1	3.577	12	0	1	0	10	0	3
495	M16	1	max	34.637	5	342.272	2	8.364	5	0	.033	1	0	2
496		min	-17.522	1	-162.101	3	-59.581	1	0	2	-.028	5	0	3
497	2	max	29.726	5	242.711	2	9.049	5	0	3	.007	1	.069	3
498		min	-17.522	1	-115.245	3	-45.018	1	0	2	-.023	5	-.146	2
499	3	max	24.816	5	143.151	2	9.733	5	0	3	0	12	.115	3
500		min	-17.522	1	-68.389	3	-30.456	1	0	2	-.021	4	-.243	2
501	4	max	19.905	5	43.591	2	10.417	5	0	3	-.001	12	.138	3
502		min	-17.522	1	-21.533	3	-15.894	1	0	2	-.024	1	-.289	2
503	5	max	14.994	5	25.322	3	11.102	5	0	3	-.002	12	.137	3
504		min	-17.522	1	-55.969	2	-1.689	3	0	2	-.028	1	-.286	2
505	6	max	10.083	5	72.178	3	14.236	4	0	3	-.002	15	.112	3
506		min	-17.522	1	-155.529	2	-1.045	3	0	2	-.025	1	-.233	2
507	7	max	5.172	5	119.034	3	27.793	1	0	3	.004	5	.065	3
508		min	-17.522	1	-255.09	2	-.401	3	0	2	-.015	1	-.131	2
509	8	max	1.773	3	165.89	3	42.355	1	0	3	.011	4	.022	2
510		min	-17.522	1	-354.65	2	.243	3	0	2	-.004	3	-.007	3
511	9	max	1.773	3	212.746	3	56.917	1	0	3	.028	1	.224	2
512		min	-17.522	1	-454.21	2	.696	12	0	2	-.003	3	-.101	3
513	10	max	20.652	5	-9.171	15	71.48	1	0	14	.06	1	.476	2
514		min	-17.522	1	-553.77	2	-2.185	3	0	2	-.003	3	-.219	3
515	11	max	15.741	5	454.21	2	5.821	5	0	2	.028	1	.224	2
516		min	-17.48	1	-212.746	3	-56.796	1	0	3	-.012	5	-.101	3
517	12	max	10.83	5	354.65	2	6.505	5	0	2	.003	2	.022	2
518		min	-17.48	1	-165.89	3	-42.233	1	0	3	-.009	5	-.007	3
519	13	max	5.919	5	255.09	2	7.189	5	0	2	0	10	.065	3
520		min	-17.48	1	-119.034	3	-27.671	1	0	3	-.015	1	-.131	2
521	14	max	1.009	5	155.529	2	7.874	5	0	2	0	12	.112	3
522		min	-17.48	1	-72.178	3	-13.109	1	0	3	-.025	1	-.233	2
523	15	max	-.26	10	55.969	2	9.094	4	0	2	.003	5	.137	3
524		min	-17.48	1	-25.322	3	-1.067	2	0	3	-.028	1	-.286	2
525	16	max	-.26	10	21.533	3	16.016	1	0	2	.007	5	.138	3
526		min	-17.48	1	-43.591	2	.237	10	0	3	-.023	1	-.289	2
527	17	max	-.26	10	68.389	3	30.578	1	0	2	.012	5	.115	3
528		min	-17.48	1	-143.151	2	1.46	10	0	3	-.012	1	-.243	2
529	18	max	-.26	10	115.245	3	45.14	1	0	2	.019	4	.069	3
530		min	-22.309	4	-242.711	2	1.917	12	0	3	-.001	10	-.146	2
531	19	max	-.26	10	162.101	3	59.702	1	0	2	.033	1	0	2
532		min	-27.219	4	-342.272	2	2.346	12	0	3	0	10	0	3
533	M15	1	max	0	.923	3	.104	3	0	1	0	1	0	1
534		min	-57.759	3	0	1	0	1	0	3	0	3	0	1
535	2	max	0	1	.821	3	.104	3	0	1	0	1	0	1
536		min	-57.813	3	0	1	0	1	0	3	0	3	0	3
537	3	max	0	1	.718	3	.104	3	0	1	0	1	0	1
538		min	-57.867	3	0	1	0	1	0	3	0	3	0	3
539	4	max	0	1	.616	3	.104	3	0	1	0	1	0	1
540		min	-57.92	3	0	1	0	1	0	3	0	3	0	3
541	5	max	0	1	.513	3	.104	3	0	1	0	1	0	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-57.974	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.41	3	.104	3	0	1	0	1	0	1
544			min	-58.028	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.308	3	.104	3	0	1	0	3	0	1
546			min	-58.082	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.205	3	.104	3	0	1	0	3	0	1
548			min	-58.136	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.103	3	.104	3	0	1	0	3	0	1
550			min	-58.19	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.104	3	0	1	0	3	0	1
552			min	-58.244	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.104	3	0	1	0	3	0	1
554			min	-58.298	3	-.103	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.104	3	0	1	0	3	0	1
556			min	-58.352	3	-.205	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.104	3	0	1	0	3	0	1
558			min	-58.406	3	-.308	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.104	3	0	1	0	3	0	1
560			min	-58.46	3	-.41	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.104	3	0	1	0	3	0	1
562			min	-58.514	3	-.513	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.104	3	0	1	0	3	0	1
564			min	-58.568	3	-.616	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.104	3	0	1	0	3	0	1
566			min	-58.622	3	-.718	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.104	3	0	1	0	3	0	1
568			min	-58.676	3	-.821	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.104	3	0	1	0	3	0	1
570			min	-58.73	3	-.923	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.134	4	.233	4	0	3	0	3	0	1
572			min	-168.798	4	0	2	-.04	3	0	1	0	4	0	1
573		2	max	0	2	1.897	4	.211	4	0	3	0	3	0	2
574			min	-168.823	4	0	2	-.04	3	0	1	0	4	0	4
575		3	max	0	2	1.66	4	.19	4	0	3	0	3	0	2
576			min	-168.848	4	0	2	-.04	3	0	1	0	4	-.001	4
577		4	max	0	2	1.423	4	.168	4	0	3	0	3	0	2
578			min	-168.872	4	0	2	-.04	3	0	1	0	1	-.002	4
579		5	max	0	2	1.185	4	.147	4	0	3	0	3	0	2
580			min	-168.897	4	0	2	-.04	3	0	1	0	1	-.002	4
581		6	max	0	2	.948	4	.125	4	0	3	0	3	0	2
582			min	-168.922	4	0	2	-.04	3	0	1	0	1	-.002	4
583		7	max	0	2	.711	4	.104	4	0	3	0	3	0	2
584			min	-168.946	4	0	2	-.04	3	0	1	0	1	-.002	4
585		8	max	0	2	.474	4	.082	4	0	3	0	5	0	2
586			min	-168.971	4	0	2	-.04	3	0	1	0	1	-.003	4
587		9	max	0	2	.237	4	.06	4	0	3	0	5	0	2
588			min	-168.996	4	0	2	-.04	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.039	1	0	3	0	5	0	2
590			min	-169.02	4	0	1	-.04	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.039	1	0	3	0	5	0	2
592			min	-169.045	4	-.237	4	-.04	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.039	1	0	3	0	5	0	2
594			min	-169.07	4	-.474	4	-.04	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.039	1	0	3	0	5	0	2
596			min	-169.094	4	-.711	4	-.04	3	0	1	0	3	-.002	4
597		14	max	.039	11	0	2	.039	1	0	3	0	5	0	2
598			min	-169.119	4	-.948	4	-.052	5	0	1	0	3	-.002	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.099	11	0	2	.039	1	0	3	0	4	0	2
600		min	-169.143	4	-1.185	4	-.073	5	0	1	0	3	-.002	4
601	16	max	.159	11	0	2	.039	1	0	3	0	4	0	2
602		min	-169.168	4	-1.423	4	-.095	5	0	1	0	3	-.002	4
603	17	max	.219	11	0	2	.039	1	0	3	0	1	0	2
604		min	-169.193	4	-1.66	4	-.116	5	0	1	0	3	-.001	4
605	18	max	.279	11	0	2	.039	1	0	3	0	1	0	2
606		min	-169.217	4	-1.897	4	-.138	5	0	1	0	3	0	4
607	19	max	.339	11	0	2	.039	1	0	3	0	1	0	1
608		min	-169.242	4	-2.134	4	-.159	5	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.005	2	.003	1	9.995e-4	5	NC	3	NC	1
2			min	-.003	3	-.004	3	-.01	5	-2.418e-4	1	5663.178	2	NC	1
3		2	max	.002	1	.005	2	.003	1	1.019e-3	5	NC	3	NC	1
4			min	-.003	3	-.004	3	-.009	5	-2.319e-4	1	6156.926	2	NC	1
5		3	max	.002	1	.004	2	.002	1	1.038e-3	5	NC	1	NC	1
6			min	-.002	3	-.004	3	-.009	5	-2.22e-4	1	6740.262	2	NC	1
7		4	max	.002	1	.004	2	.002	1	1.058e-3	5	NC	1	NC	1
8			min	-.002	3	-.004	3	-.008	5	-2.122e-4	1	7434.48	2	NC	1
9		5	max	.002	1	.004	2	.002	1	1.077e-3	5	NC	1	NC	1
10			min	-.002	3	-.004	3	-.008	5	-2.023e-4	1	8267.875	2	NC	1
11		6	max	.001	1	.003	2	.002	1	1.096e-3	5	NC	1	NC	1
12			min	-.002	3	-.004	3	-.008	5	-1.925e-4	1	9278.675	2	NC	1
13		7	max	.001	1	.003	2	.002	1	1.116e-3	5	NC	1	NC	1
14			min	-.002	3	-.003	3	-.007	5	-1.826e-4	1	NC	1	NC	1
15	8	max	.001	1	.002	2	.001	1	1.135e-3	5	NC	1	NC	1	
16		min	-.002	3	-.003	3	-.007	5	-1.728e-4	1	NC	1	NC	1	
17	9	max	.001	1	.002	2	.001	1	1.155e-3	5	NC	1	NC	1	
18		min	-.001	3	-.003	3	-.006	5	-1.629e-4	1	NC	1	NC	1	
19	10	max	.001	1	.002	2	.001	1	1.174e-3	5	NC	1	NC	1	
20		min	-.001	3	-.003	3	-.006	5	-1.531e-4	1	NC	1	NC	1	
21	11	max	0	1	.002	2	0	1	1.193e-3	5	NC	1	NC	1	
22		min	-.001	3	-.003	3	-.005	5	-1.432e-4	1	NC	1	NC	1	
23	12	max	0	1	.001	2	0	1	1.213e-3	5	NC	1	NC	1	
24		min	-.001	3	-.002	3	-.005	5	-1.333e-4	1	NC	1	NC	1	
25	13	max	0	1	0	2	0	1	1.232e-3	5	NC	1	NC	1	
26		min	0	3	-.002	3	-.004	5	-1.235e-4	1	NC	1	NC	1	
27	14	max	0	1	0	2	0	1	1.251e-3	5	NC	1	NC	1	
28		min	0	3	-.002	3	-.003	5	-1.136e-4	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	1	1.271e-3	5	NC	1	NC	1	
30		min	0	3	-.001	3	-.003	5	-1.038e-4	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.29e-3	5	NC	1	NC	1	
32		min	0	3	-.001	3	-.002	5	-9.392e-5	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.31e-3	5	NC	1	NC	1	
34		min	0	3	0	3	-.001	5	-8.407e-5	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.329e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-7.421e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.348e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-6.435e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.929e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-6.138e-4	5	NC	1	NC	1
41		2	max	0	9	0	2	.003	5	3.874e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-6.18e-4	5	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	9	0	2	.007	5	4.82e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-6.221e-4	5	NC	1	NC	1
45		4	max	0	9	0	2	.01	4	5.766e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.262e-4	5	NC	1	NC	1
47		5	max	0	9	0	2	.013	4	6.712e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.303e-4	5	NC	1	NC	1
49		6	max	0	9	0	2	.016	4	7.658e-5	1	NC	1	NC	1
50			min	0	2	-.003	3	0	1	-6.345e-4	5	NC	1	NC	1
51		7	max	0	9	0	2	.02	4	8.603e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-6.386e-4	5	NC	1	NC	1
53		8	max	0	9	0	2	.023	4	9.549e-5	1	NC	1	NC	1
54			min	0	2	-.004	3	0	9	-6.427e-4	5	NC	1	NC	1
55		9	max	0	9	0	2	.026	4	1.05e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-6.469e-4	5	NC	1	NC	1
57		10	max	0	9	.001	2	.029	4	1.144e-4	1	NC	1	NC	1
58			min	0	2	-.005	3	0	10	-6.51e-4	5	NC	1	NC	1
59		11	max	0	9	.002	2	.032	4	1.239e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-6.551e-4	5	NC	1	NC	1
61		12	max	0	9	.002	2	.035	4	1.333e-4	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-6.593e-4	5	NC	1	NC	1
63		13	max	0	9	.003	2	.038	4	1.428e-4	1	NC	1	NC	1
64			min	0	2	-.006	3	0	10	-6.634e-4	5	NC	1	NC	1
65		14	max	0	9	.003	2	.04	4	1.522e-4	1	NC	1	NC	1
66			min	0	2	-.006	3	0	10	-6.675e-4	5	NC	1	NC	1
67		15	max	0	9	.004	2	.043	4	1.617e-4	1	NC	1	NC	1
68			min	0	2	-.006	3	0	10	-6.717e-4	5	NC	1	NC	1
69		16	max	0	9	.005	2	.046	4	1.712e-4	1	NC	1	NC	1
70			min	0	2	-.006	3	0	10	-6.758e-4	5	9217.88	2	NC	1
71		17	max	0	9	.006	2	.048	4	1.806e-4	1	NC	1	NC	1
72			min	0	2	-.006	3	0	10	-6.799e-4	5	7826.044	2	NC	1
73		18	max	0	9	.007	2	.051	4	1.901e-4	1	NC	3	NC	1
74			min	0	2	-.006	3	0	10	-6.841e-4	5	6759.369	2	NC	1
75		19	max	0	9	.008	2	.053	4	1.995e-4	1	NC	3	NC	1
76			min	0	2	-.006	3	0	10	-6.882e-4	5	5932.172	2	NC	1
77	M4	1	max	.002	1	.006	2	0	10	2.28e-3	5	NC	1	NC	1
78			min	0	3	-.005	3	-.056	4	-2.149e-4	1	NC	1	343.765	4
79		2	max	.002	1	.006	2	0	10	2.28e-3	5	NC	1	NC	1
80			min	0	3	-.005	3	-.052	4	-2.149e-4	1	NC	1	374.719	4
81		3	max	.002	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
82			min	0	3	-.004	3	-.047	4	-2.149e-4	1	NC	1	411.557	4
83		4	max	.001	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
84			min	0	3	-.004	3	-.042	4	-2.149e-4	1	NC	1	455.828	4
85		5	max	.001	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
86			min	0	3	-.004	3	-.038	4	-2.149e-4	1	NC	1	509.643	4
87		6	max	.001	1	.004	2	0	10	2.28e-3	5	NC	1	NC	1
88			min	0	3	-.003	3	-.034	4	-2.149e-4	1	NC	1	575.938	4
89		7	max	.001	1	.004	2	0	10	2.28e-3	5	NC	1	NC	1
90			min	0	3	-.003	3	-.029	4	-2.149e-4	1	NC	1	658.893	4
91		8	max	.001	1	.004	2	0	10	2.28e-3	5	NC	1	NC	1
92			min	0	3	-.003	3	-.025	4	-2.149e-4	1	NC	1	764.629	4
93		9	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.021	4	-2.149e-4	1	NC	1	902.404	4
95		10	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
96			min	0	3	-.002	3	-.018	4	-2.149e-4	1	NC	1	1086.769	4
97		11	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.014	4	-2.149e-4	1	NC	1	1341.691	4
99		12	max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.002	3	-.011	4	-2.149e-4	1	NC	1	1708.972	4
101		max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.009	4	-2.149e-4	1	NC	1	2267.018	4
103		max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1
104		min	0	3	-.001	3	-.006	4	-2.149e-4	1	NC	1	3177.55	4
105		max	0	1	.001	2	0	10	2.28e-3	5	NC	1	NC	1
106		min	0	3	-.001	3	-.004	4	-2.149e-4	1	NC	1	4820.49	4
107		max	0	1	.001	2	0	10	2.28e-3	5	NC	1	NC	1
108		min	0	3	0	3	-.002	4	-2.149e-4	1	NC	1	8275.825	4
109		max	0	1	0	2	0	10	2.28e-3	5	NC	1	NC	1
110		min	0	3	0	3	-.001	4	-2.149e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.28e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-2.149e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.28e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-2.149e-4	1	NC	1	NC	1
115	M6	max	.006	1	.018	2	0	1	1.064e-3	4	NC	3	NC	1
116		min	-.008	3	-.013	3	-.009	5	-8.515e-8	2	1686.906	2	8324.059	3
117		max	.006	1	.017	2	0	1	1.083e-3	4	NC	3	NC	1
118		min	-.008	3	-.013	3	-.009	5	-8.064e-8	2	1801.101	2	8918.42	3
119		max	.006	1	.016	2	0	1	1.102e-3	4	NC	3	NC	1
120		min	-.008	3	-.012	3	-.009	5	-7.613e-8	2	1931.47	2	9614.413	3
121		max	.005	1	.014	2	0	1	1.121e-3	4	NC	3	NC	1
122		min	-.007	3	-.011	3	-.008	5	-6.992e-7	11	2081.257	2	NC	1
123		max	.005	1	.013	2	0	1	1.14e-3	4	NC	3	NC	1
124		min	-.007	3	-.011	3	-.008	5	-1.743e-6	11	2254.642	2	NC	1
125		max	.005	1	.012	2	0	1	1.159e-3	4	NC	3	NC	1
126		min	-.006	3	-.01	3	-.008	5	-3.032e-6	1	2457.096	2	NC	1
127		max	.004	1	.011	2	0	1	1.178e-3	4	NC	3	NC	1
128		min	-.006	3	-.009	3	-.007	5	-5.382e-6	1	2695.92	2	NC	1
129		max	.004	1	.01	2	0	1	1.197e-3	4	NC	3	NC	1
130		min	-.005	3	-.009	3	-.007	5	-7.733e-6	1	2981.079	2	NC	1
131		max	.004	1	.009	2	0	1	1.215e-3	4	NC	3	NC	1
132		min	-.005	3	-.008	3	-.006	5	-1.008e-5	1	3326.531	2	NC	1
133		max	.003	1	.008	2	0	1	1.234e-3	4	NC	3	NC	1
134		min	-.004	3	-.007	3	-.006	5	-1.243e-5	1	3752.444	2	NC	1
135		max	.003	1	.007	2	0	1	1.253e-3	4	NC	3	NC	1
136		min	-.004	3	-.007	3	-.005	5	-1.478e-5	1	4289.076	2	NC	1
137		max	.002	1	.006	2	0	1	1.272e-3	4	NC	3	NC	1
138		min	-.003	3	-.006	3	-.005	5	-1.713e-5	1	4983.983	2	NC	1
139		max	.002	1	.005	2	0	1	1.291e-3	4	NC	3	NC	1
140		min	-.003	3	-.005	3	-.004	5	-1.948e-5	1	5916.431	2	NC	1
141		max	.002	1	.004	2	0	1	1.31e-3	4	NC	3	NC	1
142		min	-.002	3	-.004	3	-.003	5	-2.183e-5	1	7229.102	2	NC	1
143		max	.001	1	.003	2	0	1	1.329e-3	4	NC	1	NC	1
144		min	-.002	3	-.003	3	-.003	5	-2.418e-5	1	9207.366	2	NC	1
145		max	.001	1	.002	2	0	1	1.348e-3	4	NC	1	NC	1
146		min	-.001	3	-.003	3	-.002	5	-2.654e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.367e-3	4	NC	1	NC	1
148		min	0	3	-.002	3	-.001	5	-2.889e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.386e-3	4	NC	1	NC	1
150		min	0	3	0	3	0	5	-3.124e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.404e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.359e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.521e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.393e-4	4	NC	1	NC	1
155		max	0	9	.001	2	.003	4	1.39e-5	1	NC	1	NC	1
156		min	0	2	-.001	3	0	1	-6.303e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157	3	max	0	9	.002	2	.007	4	1.259e-5	1	NC	1	NC	1
158		min	0	2	-.003	3	0	1	-6.212e-4	4	NC	1	NC	1
159	4	max	0	9	.003	2	.01	4	1.128e-5	1	NC	1	NC	1
160		min	0	2	-.004	3	0	1	-6.122e-4	4	NC	1	NC	1
161	5	max	0	9	.004	2	.014	4	9.975e-6	1	NC	1	NC	1
162		min	0	2	-.006	3	0	1	-6.032e-4	4	NC	1	NC	1
163	6	max	0	9	.005	2	.017	4	8.666e-6	1	NC	1	NC	1
164		min	0	2	-.007	3	0	1	-5.942e-4	4	8553.554	2	NC	1
165	7	max	0	9	.007	2	.021	4	2.283e-5	3	NC	3	NC	1
166		min	0	2	-.008	3	0	1	-5.852e-4	4	7075.993	2	NC	1
167	8	max	0	9	.008	2	.024	4	3.846e-5	3	NC	3	NC	1
168		min	0	2	-.01	3	0	1	-5.762e-4	4	5988.877	2	NC	1
169	9	max	0	9	.009	2	.027	4	5.409e-5	3	NC	3	NC	1
170		min	0	2	-.011	3	0	1	-5.672e-4	4	5151.328	2	NC	1
171	10	max	0	9	.01	2	.03	4	6.972e-5	3	NC	3	NC	1
172		min	-.001	2	-.012	3	0	1	-5.582e-4	4	4485.159	2	NC	1
173	11	max	0	9	.012	2	.033	4	8.535e-5	3	NC	3	NC	1
174		min	-.001	2	-.013	3	0	1	-5.492e-4	4	3943.235	2	NC	1
175	12	max	0	9	.013	2	.036	4	1.01e-4	3	NC	3	NC	1
176		min	-.001	2	-.014	3	0	1	-5.402e-4	4	3495.187	2	NC	1
177	13	max	0	9	.015	2	.039	4	1.166e-4	3	NC	3	NC	1
178		min	-.001	2	-.015	3	0	1	-5.312e-4	4	3120.341	2	NC	1
179	14	max	0	9	.016	2	.042	4	1.322e-4	3	NC	3	NC	1
180		min	-.002	2	-.016	3	0	1	-5.222e-4	4	2803.965	2	NC	1
181	15	max	0	9	.018	2	.045	4	1.479e-4	3	NC	3	NC	1
182		min	-.002	2	-.017	3	0	1	-5.132e-4	4	2535.166	2	NC	1
183	16	max	0	9	.02	2	.047	4	1.635e-4	3	NC	3	NC	1
184		min	-.002	2	-.018	3	0	1	-5.042e-4	4	2305.65	2	NC	1
185	17	max	.001	9	.022	2	.05	4	1.791e-4	3	NC	3	NC	1
186		min	-.002	2	-.018	3	0	1	-4.952e-4	4	2108.956	2	NC	1
187	18	max	.001	9	.024	2	.052	4	1.948e-4	3	NC	3	NC	1
188		min	-.002	2	-.019	3	0	1	-4.862e-4	4	1939.966	2	NC	1
189	19	max	.001	9	.026	2	.055	4	2.104e-4	3	NC	3	NC	1
190		min	-.002	2	-.02	3	0	1	-4.771e-4	4	1794.578	2	NC	1
191	M8	1	max	.005	1	.02	2	0	2.095e-3	4	NC	1	NC	1
192		min	-.002	3	-.015	3	-.057	4	-1.674e-4	3	NC	1	336.691	4
193	2	max	.005	1	.019	2	0	1	2.095e-3	4	NC	1	NC	1
194		min	-.002	3	-.014	3	-.053	4	-1.674e-4	3	NC	1	367.008	4
195	3	max	.005	1	.018	2	0	1	2.095e-3	4	NC	1	NC	1
196		min	-.002	3	-.013	3	-.048	4	-1.674e-4	3	NC	1	403.089	4
197	4	max	.004	1	.017	2	0	1	2.095e-3	4	NC	1	NC	1
198		min	-.002	3	-.012	3	-.043	4	-1.674e-4	3	NC	1	446.451	4
199	5	max	.004	1	.016	2	0	1	2.095e-3	4	NC	1	NC	1
200		min	-.001	3	-.012	3	-.039	4	-1.674e-4	3	NC	1	499.162	4
201	6	max	.004	1	.015	2	0	1	2.095e-3	4	NC	1	NC	1
202		min	-.001	3	-.011	3	-.034	4	-1.674e-4	3	NC	1	564.096	4
203	7	max	.004	1	.014	2	0	1	2.095e-3	4	NC	1	NC	1
204		min	-.001	3	-.01	3	-.03	4	-1.674e-4	3	NC	1	645.349	4
205	8	max	.003	1	.012	2	0	1	2.095e-3	4	NC	1	NC	1
206		min	-.001	3	-.009	3	-.026	4	-1.674e-4	3	NC	1	748.916	4
207	9	max	.003	1	.011	2	0	1	2.095e-3	4	NC	1	NC	1
208		min	-.001	3	-.008	3	-.022	4	-1.674e-4	3	NC	1	883.864	4
209	10	max	.003	1	.01	2	0	1	2.095e-3	4	NC	1	NC	1
210		min	0	3	-.007	3	-.018	4	-1.674e-4	3	NC	1	1064.449	4
211	11	max	.002	1	.009	2	0	1	2.095e-3	4	NC	1	NC	1
212		min	0	3	-.007	3	-.015	4	-1.674e-4	3	NC	1	1314.145	4
213	12	max	.002	1	.008	2	0	1	2.095e-3	4	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.006	3	-.012	4	-1.674e-4	3	NC	1	1673.898	4
215		13	max	.002	1	.007	2	0	1	2.095e-3	4	NC	1	NC	1
216			min	0	3	-.005	3	-.009	4	-1.674e-4	3	NC	1	2220.508	4
217		14	max	.001	1	.006	2	0	1	2.095e-3	4	NC	1	NC	1
218			min	0	3	-.004	3	-.006	4	-1.674e-4	3	NC	1	3112.384	4
219		15	max	.001	1	.005	2	0	1	2.095e-3	4	NC	1	NC	1
220			min	0	3	-.003	3	-.004	4	-1.674e-4	3	NC	1	4721.671	4
221		16	max	0	1	.003	2	0	1	2.095e-3	4	NC	1	NC	1
222			min	0	3	-.002	3	-.002	4	-1.674e-4	3	NC	1	8106.24	4
223		17	max	0	1	.002	2	0	1	2.095e-3	4	NC	1	NC	1
224			min	0	3	-.002	3	-.001	4	-1.674e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	2.095e-3	4	NC	1	NC	1
226			min	0	3	0	3	0	4	-1.674e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	2.095e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.674e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.005	2	0	3	2.575e-4	1	NC	3	NC	1
230			min	-.002	3	-.004	3	-.004	4	-3.492e-4	3	5674.732	2	NC	1
231		2	max	.002	1	.005	2	0	3	2.45e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.004	4	-3.393e-4	3	6169.746	2	NC	1
233		3	max	.002	1	.004	2	0	3	2.837e-4	4	NC	1	NC	1
234			min	-.002	3	-.004	3	-.004	4	-3.295e-4	3	6754.625	2	NC	1
235		4	max	.002	1	.004	2	0	3	3.3e-4	4	NC	1	NC	1
236			min	-.002	3	-.004	3	-.004	4	-3.197e-4	3	7450.741	2	NC	1
237		5	max	.002	1	.004	2	0	3	3.764e-4	4	NC	1	NC	1
238			min	-.002	3	-.004	3	-.004	4	-3.098e-4	3	8286.493	2	NC	1
239		6	max	.001	1	.003	2	0	3	4.227e-4	4	NC	1	NC	1
240			min	-.002	3	-.004	3	-.004	4	-3.e-4	3	9300.257	2	NC	1
241		7	max	.001	1	.003	2	0	3	4.69e-4	4	NC	1	NC	1
242			min	-.002	3	-.004	3	-.004	4	-2.901e-4	3	NC	1	NC	1
243		8	max	.001	1	.002	2	0	3	5.154e-4	4	NC	1	NC	1
244			min	-.001	3	-.003	3	-.004	4	-2.803e-4	3	NC	1	NC	1
245		9	max	.001	1	.002	2	0	3	5.617e-4	4	NC	1	NC	1
246			min	-.001	3	-.003	3	-.003	4	-2.705e-4	3	NC	1	NC	1
247		10	max	.001	1	.002	2	0	3	6.08e-4	4	NC	1	NC	1
248			min	-.001	3	-.003	3	-.003	4	-2.606e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	6.544e-4	4	NC	1	NC	1
250			min	-.001	3	-.003	3	-.003	4	-2.508e-4	3	NC	1	NC	1
251		12	max	0	1	.001	2	0	3	7.007e-4	4	NC	1	NC	1
252			min	0	3	-.002	3	-.003	4	-2.41e-4	3	NC	1	NC	1
253		13	max	0	1	0	2	0	3	7.47e-4	4	NC	1	NC	1
254			min	0	3	-.002	3	-.003	4	-2.311e-4	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	7.933e-4	4	NC	1	NC	1
256			min	0	3	-.002	3	-.002	4	-2.213e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	8.397e-4	4	NC	1	NC	1
258			min	0	3	-.002	3	-.002	4	-2.115e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	8.86e-4	4	NC	1	NC	1
260			min	0	3	-.001	3	-.001	4	-2.016e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	9.323e-4	4	NC	1	NC	1
262			min	0	3	0	3	0	4	-1.918e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	9.787e-4	4	NC	1	NC	1
264			min	0	3	0	3	0	4	-1.82e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.025e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.721e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.856e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-4.672e-4	4	NC	1	NC	1
269		2	max	0	9	0	2	.003	4	6.3e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-5.17e-4	4	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	9	0	2	.005	4	4.744e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-5.668e-4	4	NC	1	9037.956	4
273		4	max	0	9	0	2	.008	4	3.188e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-6.166e-4	4	NC	1	5957.257	4
275		5	max	0	9	0	2	.01	4	1.632e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-6.664e-4	4	NC	1	4428.822	4
277		6	max	0	9	0	2	.013	4	7.595e-7	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-7.163e-4	4	NC	1	3519.844	4
279		7	max	0	9	0	2	.016	4	-1.286e-6	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-7.661e-4	4	NC	1	2919.538	4
281		8	max	0	9	0	2	.018	5	-1.418e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-8.159e-4	4	NC	1	2494.783	5
283		9	max	0	9	0	2	.021	5	-1.551e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	3	-8.657e-4	4	NC	1	2176.64	5
285		10	max	0	9	.001	2	.024	5	-1.683e-6	10	NC	1	NC	1
286			min	0	2	-.005	3	-.002	3	-9.156e-4	4	NC	1	1931.436	5
287		11	max	0	9	.002	2	.026	5	-1.815e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.002	3	-9.654e-4	4	NC	1	1736.797	5
289		12	max	0	9	.002	2	.029	5	-1.947e-6	10	NC	1	NC	1
290			min	0	2	-.006	3	-.002	3	-1.015e-3	4	NC	1	1578.494	5
291		13	max	0	9	.003	2	.032	5	-2.079e-6	10	NC	1	NC	1
292			min	0	2	-.006	3	-.002	3	-1.065e-3	4	NC	1	1447.049	5
293		14	max	0	9	.003	2	.034	5	-2.212e-6	10	NC	1	NC	1
294			min	0	2	-.006	3	-.002	3	-1.115e-3	4	NC	1	1335.899	5
295		15	max	0	9	.004	2	.037	5	-2.344e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.002	3	-1.165e-3	4	NC	1	1240.359	5
297		16	max	0	9	.005	2	.04	5	-2.476e-6	10	NC	1	NC	1
298			min	0	2	-.007	3	-.002	1	-1.215e-3	4	9230.757	2	1156.991	5
299		17	max	0	9	.006	2	.042	5	-2.608e-6	10	NC	1	NC	1
300			min	0	2	-.007	3	-.003	1	-1.264e-3	4	7835.722	2	1083.222	5
301		18	max	0	9	.007	2	.045	5	-2.741e-6	10	NC	3	NC	1
302			min	0	2	-.006	3	-.003	1	-1.314e-3	4	6766.879	2	1017.086	5
303		19	max	0	9	.008	2	.048	5	-2.873e-6	10	NC	3	NC	1
304			min	0	2	-.006	3	-.003	1	-1.364e-3	4	5938.178	2	957.069	5
305	M12	1	max	.002	1	.006	2	.003	1	2.846e-3	4	NC	1	NC	2
306			min	0	3	-.005	3	-.053	5	2.593e-6	10	NC	1	366.538	5
307		2	max	.002	1	.006	2	.002	1	2.846e-3	4	NC	1	NC	2
308			min	0	3	-.005	3	-.048	5	2.593e-6	10	NC	1	399.534	5
309		3	max	.002	1	.005	2	.002	1	2.846e-3	4	NC	1	NC	2
310			min	0	3	-.004	3	-.044	5	2.593e-6	10	NC	1	438.802	5
311		4	max	.001	1	.005	2	.002	1	2.846e-3	4	NC	1	NC	1
312			min	0	3	-.004	3	-.04	5	2.593e-6	10	NC	1	485.992	5
313		5	max	.001	1	.005	2	.002	1	2.846e-3	4	NC	1	NC	1
314			min	0	3	-.004	3	-.036	5	2.593e-6	10	NC	1	543.355	5
315		6	max	.001	1	.004	2	.001	1	2.846e-3	4	NC	1	NC	1
316			min	0	3	-.003	3	-.031	5	2.593e-6	10	NC	1	614.019	5
317		7	max	.001	1	.004	2	.001	1	2.846e-3	4	NC	1	NC	1
318			min	0	3	-.003	3	-.028	5	2.593e-6	10	NC	1	702.44	5
319		8	max	.001	1	.004	2	.001	1	2.846e-3	4	NC	1	NC	1
320			min	0	3	-.003	3	-.024	5	2.593e-6	10	NC	1	815.141	5
321		9	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
322			min	0	3	-.003	3	-.02	5	2.593e-6	10	NC	1	961.989	5
323		10	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
324			min	0	3	-.002	3	-.017	5	2.593e-6	10	NC	1	1158.492	5
325		11	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
326			min	0	3	-.002	3	-.014	5	2.593e-6	10	NC	1	1430.193	5
327		12	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.011	5	2.593e-6	10	NC	1	1821.641	5
329		13	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.008	5	2.593e-6	10	NC	1	2416.397	5
331		14	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1
332			min	0	3	-.001	3	-.006	5	2.593e-6	10	NC	1	3386.81	5
333		15	max	0	1	.001	2	0	1	2.846e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.004	5	2.593e-6	10	NC	1	5137.769	5
335		16	max	0	1	.001	2	0	1	2.846e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.002	5	2.593e-6	10	NC	1	8820.212	5
337		17	max	0	1	0	2	0	1	2.846e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.001	5	2.593e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.846e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	2.593e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.846e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	2.593e-6	10	NC	1	NC	1
343	M1	1	max	.004	3	.021	3	.005	5	8.312e-3	1	NC	1	NC	1
344			min	-.005	2	-.019	1	0	1	-1.009e-2	3	NC	1	NC	1
345		2	max	.004	3	.011	3	.007	5	4.053e-3	1	NC	4	NC	1
346			min	-.005	2	-.01	1	-.002	1	-4.968e-3	3	5122.109	3	NC	1
347		3	max	.004	3	.002	3	.01	5	1.943e-4	5	NC	4	NC	1
348			min	-.005	2	-.002	1	-.003	1	-1.265e-4	1	2659.86	3	9993.366	5
349		4	max	.004	3	.006	2	.012	5	1.873e-4	5	NC	4	NC	1
350			min	-.005	2	-.005	3	-.003	1	-1.022e-4	1	1884.142	1	6294.748	5
351		5	max	.004	3	.012	2	.015	5	1.803e-4	5	NC	4	NC	1
352			min	-.005	2	-.011	3	-.003	1	-7.791e-5	1	1503.421	2	4497.945	5
353		6	max	.004	3	.017	2	.018	5	1.732e-4	5	NC	4	NC	1
354			min	-.005	2	-.016	3	-.003	1	-5.361e-5	1	1281.902	2	3452.202	5
355		7	max	.004	3	.021	2	.022	5	1.662e-4	5	NC	5	NC	1
356			min	-.005	2	-.019	3	-.003	1	-2.93e-5	1	1145.881	2	2776.777	5
357		8	max	.004	3	.024	2	.025	5	1.592e-4	5	NC	5	NC	1
358			min	-.005	2	-.022	3	-.002	1	-1.195e-5	9	1061.25	2	2309.717	5
359		9	max	.004	3	.026	2	.029	5	1.551e-4	4	NC	5	NC	1
360			min	-.005	2	-.023	3	-.002	1	1.02e-6	10	1011.944	2	1965.934	4
361		10	max	.004	3	.027	2	.032	4	1.534e-4	4	NC	5	NC	1
362			min	-.005	2	-.023	3	0	1	1.302e-6	10	990.355	2	1700.479	4
363		11	max	.004	3	.026	2	.036	4	1.517e-4	4	NC	5	NC	1
364			min	-.005	2	-.022	3	0	9	1.583e-6	10	993.826	2	1497.669	4
365		12	max	.004	3	.025	2	.04	4	1.5e-4	4	NC	5	NC	1
366			min	-.005	2	-.02	3	0	10	1.864e-6	10	1023.64	2	1339.344	4
367		13	max	.004	3	.022	2	.043	4	1.484e-4	4	NC	5	NC	1
368			min	-.005	2	-.018	3	0	10	2.146e-6	10	1085.661	2	1213.681	4
369		14	max	.004	3	.018	2	.047	4	1.467e-4	4	NC	4	NC	1
370			min	-.005	2	-.014	3	0	10	2.427e-6	10	1193.239	2	1112.691	4
371		15	max	.004	3	.012	2	.05	4	1.651e-4	1	NC	4	NC	1
372			min	-.005	2	-.01	3	0	10	2.709e-6	10	1375.456	2	1030.825	4
373		16	max	.005	3	.005	2	.053	4	3.188e-4	4	NC	4	NC	1
374			min	-.005	2	-.004	3	0	10	2.939e-6	10	1702.666	2	964.128	4
375		17	max	.005	3	.002	3	.056	4	4.666e-3	4	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	1.945e-6	10	2393.406	2	909.779	4
377		18	max	.005	3	.008	3	.059	4	5.173e-3	2	NC	4	NC	1
378			min	-.005	2	-.013	2	0	10	-2.531e-3	3	4623.872	2	865.535	4
379		19	max	.005	3	.015	3	.061	4	1.042e-2	2	NC	1	NC	1
380			min	-.005	2	-.023	2	0	1	-5.146e-3	3	NC	1	830.737	4
381	M5	1	max	.014	3	.066	3	.005	5	8.041e-6	4	NC	1	NC	1
382			min	-.018	2	-.062	1	0	1	0	1	NC	1	NC	1
383		2	max	.014	3	.036	3	.007	5	9.064e-5	5	NC	4	NC	1
384			min	-.018	2	-.033	1	0	1	-1.977e-5	1	1577.388	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.014	3	.008	3	.009	5	1.719e-4	5	NC	5	NC	1
386		min	-.018	2	-.005	1	0	1	-3.916e-5	1	812.841	1	NC	1
387	4	max	.014	3	.019	2	.012	5	1.789e-4	5	NC	5	NC	1
388		min	-.018	2	-.016	3	0	1	-3.646e-5	1	573.227	1	NC	1
389	5	max	.014	3	.039	2	.016	5	1.859e-4	5	NC	5	NC	1
390		min	-.018	2	-.035	3	0	1	-3.425e-5	9	457.905	1	NC	1
391	6	max	.014	3	.055	2	.019	5	1.929e-4	5	NC	5	NC	1
392		min	-.018	2	-.05	3	0	1	-3.207e-5	9	392.563	1	NC	1
393	7	max	.014	3	.069	2	.023	5	1.999e-4	5	NC	5	NC	1
394		min	-.018	2	-.061	3	0	1	-2.988e-5	9	352.324	2	NC	1
395	8	max	.014	3	.078	2	.027	5	2.069e-4	5	NC	5	NC	1
396		min	-.018	2	-.069	3	0	1	-2.77e-5	9	326.181	2	NC	1
397	9	max	.014	3	.085	2	.03	4	2.138e-4	5	NC	5	NC	1
398		min	-.018	2	-.073	3	0	1	-2.551e-5	9	310.93	2	NC	1
399	10	max	.014	3	.087	2	.034	4	2.208e-4	5	NC	5	NC	1
400		min	-.018	2	-.073	3	0	1	-2.333e-5	9	304.221	2	NC	1
401	11	max	.014	3	.086	2	.038	4	2.278e-4	5	NC	5	NC	1
402		min	-.018	2	-.07	3	0	1	-2.114e-5	9	305.232	2	NC	1
403	12	max	.014	3	.08	2	.042	4	2.348e-4	5	NC	5	NC	1
404		min	-.018	2	-.065	3	0	1	-1.896e-5	9	314.352	2	NC	1
405	13	max	.014	3	.071	2	.045	4	2.418e-4	5	NC	5	NC	1
406		min	-.018	2	-.056	3	0	1	-1.677e-5	9	333.382	2	NC	1
407	14	max	.014	3	.057	2	.049	4	2.488e-4	5	NC	5	NC	1
408		min	-.018	2	-.044	3	0	1	-1.459e-5	9	366.429	2	NC	1
409	15	max	.014	3	.039	2	.052	4	2.562e-4	4	NC	5	NC	1
410		min	-.018	2	-.03	3	0	1	-1.24e-5	9	422.442	2	NC	1
411	16	max	.014	3	.017	2	.055	4	4.364e-4	4	NC	5	NC	1
412		min	-.018	2	-.014	3	0	1	-1.124e-5	9	523.089	2	NC	1
413	17	max	.014	3	.005	3	.057	4	4.714e-3	4	NC	5	NC	1
414		min	-.018	2	-.01	2	0	1	-3.846e-5	1	735.861	2	NC	1
415	18	max	.014	3	.026	3	.059	4	2.421e-3	4	NC	4	NC	1
416		min	-.018	2	-.041	2	0	1	-1.976e-5	1	1422.24	2	NC	1
417	19	max	.014	3	.049	3	.061	4	3.48e-6	5	NC	1	NC	1
418		min	-.018	2	-.075	2	0	1	-3.32e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.02	.004	5	1.01e-2	3	NC	1	NC	1
420		min	-.005	2	-.019	1	-.001	1	-8.312e-3	1	NC	1	NC	1
421	2	max	.005	3	.011	3	.004	5	5.017e-3	3	NC	4	NC	1
422		min	-.005	2	-.01	1	0	9	-4.098e-3	1	5124.207	3	NC	1
423	3	max	.004	3	.002	3	.004	4	3.737e-5	1	NC	4	NC	1
424		min	-.005	2	-.002	1	0	3	-3.524e-5	5	2660.98	3	NC	1
425	4	max	.005	3	.006	2	.005	4	1.974e-5	3	NC	4	NC	1
426		min	-.005	2	-.005	3	-.001	3	-5.203e-5	5	1885.39	1	NC	1
427	5	max	.005	3	.012	2	.006	4	1.079e-5	2	NC	4	NC	1
428		min	-.005	2	-.011	3	-.002	3	-7.205e-5	4	1503.718	2	NC	1
429	6	max	.005	3	.017	2	.008	4	4.546e-6	2	NC	4	NC	1
430		min	-.005	2	-.016	3	-.002	3	-9.274e-5	4	1282.168	2	NC	1
431	7	max	.005	3	.021	2	.011	4	-3.2e-7	10	NC	5	NC	1
432		min	-.005	2	-.019	3	-.003	3	-1.134e-4	4	1146.129	2	6640.443	4
433	8	max	.005	3	.024	2	.014	4	-6.093e-7	10	NC	5	NC	1
434		min	-.005	2	-.022	3	-.003	3	-1.341e-4	4	1061.49	2	4610.767	4
435	9	max	.005	3	.026	2	.017	4	-8.987e-7	10	NC	5	NC	1
436		min	-.005	2	-.023	3	-.003	3	-1.548e-4	4	1012.182	2	3428.712	4
437	10	max	.005	3	.027	2	.021	5	-1.188e-6	10	NC	5	NC	1
438		min	-.005	2	-.023	3	-.003	3	-1.755e-4	4	990.596	2	2676.895	4
439	11	max	.005	3	.026	2	.026	5	-1.477e-6	10	NC	5	NC	1
440		min	-.005	2	-.022	3	-.003	3	-1.962e-4	4	994.075	2	2167.787	4
441	12	max	.005	3	.025	2	.03	5	-1.767e-6	10	NC	5	NC	1









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E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.