

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.026 k-ft
$P_n$ =	0.141 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

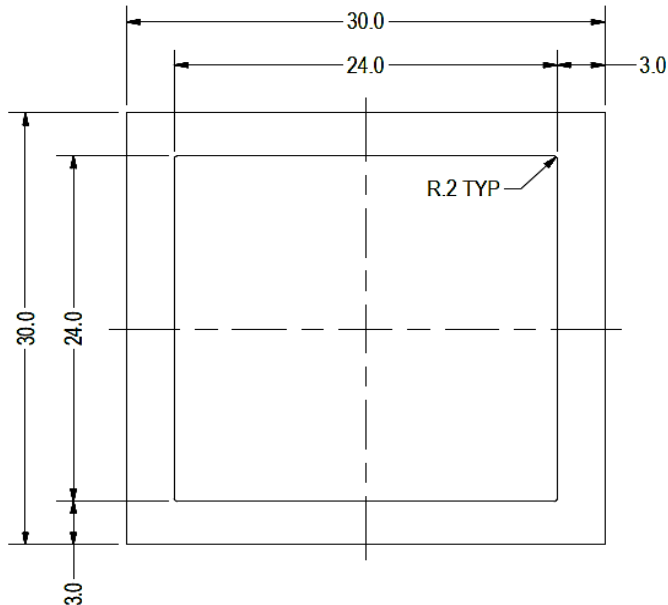
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.601 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

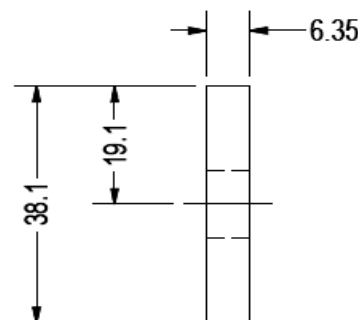
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.635 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>13%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.174 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

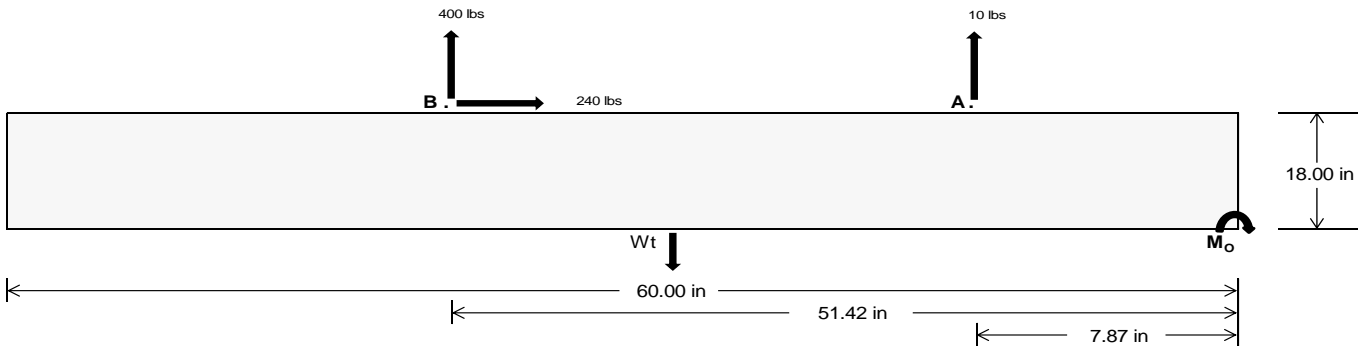
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>48.58</b>	<b>1735.74</b>	k
Compressive Load =	<b>978.93</b>	<b>1142.28</b>	k
Lateral Load =	<b>21.68</b>	<b>1041.87</b>	k
Moment (Weak Axis) =	<b>0.03</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 24964.1$  in-lbs  
Resisting Force Required = 832.14 lbs  
S.F. = 1.67  
Weight Required = 1386.90 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 240.35 lbs  
Friction = 0.4  
Weight Required = 600.88 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 240.35 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	322 lbs	322 lbs	322 lbs	322 lbs	373 lbs	373 lbs	373 lbs	373 lbs	492 lbs	492 lbs	492 lbs	492 lbs	-20 lbs	-20 lbs	-20 lbs	-20 lbs
$F_B$	217 lbs	217 lbs	217 lbs	217 lbs	486 lbs	486 lbs	486 lbs	486 lbs	507 lbs	507 lbs	507 lbs	507 lbs	-800 lbs	-800 lbs	-800 lbs	-800 lbs
$F_V$	29 lbs	29 lbs	29 lbs	29 lbs	430 lbs	430 lbs	430 lbs	430 lbs	342 lbs	342 lbs	342 lbs	342 lbs	-481 lbs	-481 lbs	-481 lbs	-481 lbs
$P_{total}$	2443 lbs	2533 lbs	2624 lbs	2714 lbs	2762 lbs	2853 lbs	2944 lbs	3034 lbs	2902 lbs	2992 lbs	3083 lbs	3173 lbs	322 lbs	377 lbs	431 lbs	485 lbs
$M$	250 lbs-ft	250 lbs-ft	250 lbs-ft	250 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	516 lbs-ft	516 lbs-ft	516 lbs-ft	516 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft
$e$	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.08 ft	1.78 ft	1.55 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	244.8 psf	243.6 psf	242.4 psf	241.4 psf	251.8 psf	250.3 psf	248.9 psf	247.5 psf	260.9 psf	258.9 psf	257.1 psf	255.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	313.5 psf	309.1 psf	305.2 psf	301.5 psf	379.6 psf	372.2 psf	365.5 psf	359.3 psf	402.3 psf	393.9 psf	386.3 psf	379.2 psf	290.5 psf	189.5 psf	158.3 psf	144.3 psf

Maximum Bearing Pressure = 402 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

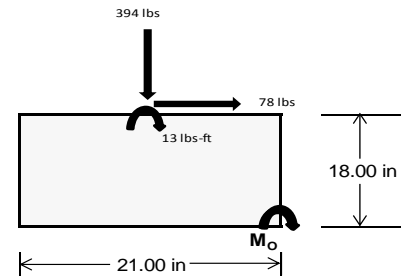
### Overturning Check

$M_o = 215.0 \text{ ft-lbs}$   
 Resisting Force Required = 245.70 lbs  
 S.F. = 1.67  
 Weight Required = 409.50 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	118 lbs	54 lbs	56 lbs	211 lbs	394 lbs	164 lbs	81 lbs	-35 lbs	21 lbs
$F_v$	13 lbs	104 lbs	13 lbs	9 lbs	78 lbs	10 lbs	13 lbs	104 lbs	13 lbs
$P_{total}$	2474 lbs	2410 lbs	2412 lbs	2454 lbs	2637 lbs	2406 lbs	770 lbs	654 lbs	710 lbs
$M$	35 lbs-ft	174 lbs-ft	36 lbs-ft	25 lbs-ft	130 lbs-ft	27 lbs-ft	35 lbs-ft	173 lbs-ft	36 lbs-ft
$e$	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.27 ft	0.05 ft
$L/6$	0.29 ft	1.61 ft	1.72 ft	1.73 ft	1.65 ft	1.73 ft	1.66 ft	1.22 ft	1.65 ft
$f_{min}$	269.1 sqft	207.4 sqft	261.6 sqft	270.5 sqft	250.4 sqft	264.3 sqft	74.2 sqft	6.8 sqft	67.1 sqft
$f_{max}$	296.4 psf	343.5 psf	289.7 psf	290.4 psf	352.3 psf	285.8 psf	101.7 psf	142.7 psf	95.1 psf



Maximum Bearing Pressure = 352 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.765 k
Allowable Uplift =	1.214 k
Utilization =	<u>63%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.095 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.753 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

#### Diagonal Strut

Maximum Axial Load =	0.601 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.098 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.174 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.054 in
	<u>0.054 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$124.989$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$129.794$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.266 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	203.988	2	-.016	15	.111	1	0	10	0	4	0	15
30		min	-362.085	3	-.074	3	-.556	5	0	4	0	3	0	6
31	16	max	204.114	2	-.028	15	.111	1	0	10	0	4	0	15
32		min	-361.99	3	-.114	4	-.67	5	0	4	0	3	0	6
33	17	max	204.24	2	-.04	15	.111	1	0	10	0	4	0	15
34		min	-361.896	3	-.165	4	-.784	5	0	4	0	3	0	6
35	18	max	204.365	2	-.052	15	.111	1	0	10	0	1	0	15
36		min	-361.802	3	-.216	4	-.899	5	0	4	0	3	0	6
37	19	max	204.491	2	-.064	15	.111	1	0	10	0	1	0	15
38		min	-361.707	3	-.267	4	-1.013	5	0	4	0	3	0	6
39	M3	1	max	187.255	2	1.757	.002	10	0	5	0	4	0	6
40		min	-177.951	3	.412	15	-1.329	4	0	1	0	10	0	15
41	2	max	187.186	2	1.58	6	-.002	10	0	5	0	1	0	2
42		min	-178.003	3	.371	15	-1.196	4	0	1	0	10	0	12
43	3	max	187.116	2	1.403	6	.002	10	0	5	0	1	0	2
44		min	-178.055	3	.329	15	-1.062	4	0	1	0	5	0	3
45	4	max	187.047	2	1.226	6	.002	10	0	5	0	1	0	15
46		min	-178.107	3	.287	15	-.928	4	0	1	0	5	0	4
47	5	max	186.978	2	1.049	6	.002	10	0	5	0	1	0	15
48		min	-178.159	3	.246	15	-.795	4	0	1	0	5	0	4
49	6	max	186.908	2	.872	6	.002	10	0	5	0	1	0	15
50		min	-178.211	3	.204	15	-.661	4	0	1	0	5	0	4
51	7	max	186.839	2	.696	6	-.002	10	0	5	0	1	0	15
52		min	-178.263	3	.163	15	-.527	4	0	1	0	5	0	4
53	8	max	186.77	2	.519	6	.002	10	0	5	0	1	0	15
54		min	-178.315	3	.121	15	-.394	4	0	1	0	5	-.001	4
55	9	max	186.7	2	.342	6	.002	10	0	5	0	1	0	15
56		min	-178.367	3	.08	15	-.26	4	0	1	0	5	-.001	4
57	10	max	186.631	2	.165	6	.002	10	0	5	0	1	0	15
58		min	-178.419	3	.038	15	-.145	1	0	1	0	5	-.001	4
59	11	max	186.562	2	.019	2	.042	5	0	5	0	1	0	15
60		min	-178.471	3	-.038	3	-.145	1	0	1	0	5	-.001	4
61	12	max	186.492	2	-.045	15	.176	5	0	5	0	1	0	15
62		min	-178.523	3	-.189	4	-.145	1	0	1	0	5	-.001	4
63	13	max	186.423	2	-.087	15	.31	5	0	5	0	1	0	15
64		min	-178.575	3	-.365	4	-.145	1	0	1	0	5	-.001	4
65	14	max	186.354	2	-.128	15	.443	5	0	5	0	1	0	15
66		min	-178.627	3	-.542	4	-.145	1	0	1	0	5	-.001	4
67	15	max	186.284	2	-.17	15	.577	5	0	5	0	1	0	15
68		min	-178.679	3	-.719	4	-.145	1	0	1	0	5	0	4
69	16	max	186.215	2	-.211	15	.711	5	0	5	0	9	0	15
70		min	-178.731	3	-.896	4	-.145	1	0	1	0	5	0	4
71	17	max	186.146	2	-.253	15	.844	5	0	5	0	10	0	15
72		min	-178.783	3	-1.073	4	-.145	1	0	1	0	4	0	4
73	18	max	186.076	2	-.295	15	.978	5	0	5	0	10	0	15
74		min	-178.835	3	-1.25	4	-.145	1	0	1	0	4	0	4
75	19	max	186.007	2	-.336	15	1.112	5	0	5	0	5	0	1
76		min	-178.887	3	-1.426	4	-.145	1	0	1	0	1	0	1
77	M4	1	max	270.469	1	0	.016	10	0	1	0	5	0	1
78		min	2.704	12	0	1	-15.521	4	0	1	0	2	0	1
79	2	max	270.534	1	0	1	.016	10	0	1	0	10	0	1
80		min	2.737	12	0	1	-15.577	4	0	1	-.001	4	0	1
81	3	max	270.599	1	0	1	.016	10	0	1	0	10	0	1
82		min	2.769	12	0	1	-15.633	4	0	1	-.003	4	0	1
83	4	max	270.663	1	0	1	.016	10	0	1	0	10	0	1
84		min	2.801	12	0	1	-15.689	4	0	1	-.004	4	0	1
85	5	max	270.728	1	0	1	.016	10	0	1	0	10	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	2.834	12	0	1	-15.745	4	0	1	-.006	4	0	1
87		6	max	270.793	1	0	1	.016	10	0	1	0	10	0	1
88			min	2.866	12	0	1	-15.801	4	0	1	-.007	4	0	1
89		7	max	270.857	1	0	1	.016	10	0	1	0	10	0	1
90			min	2.898	12	0	1	-15.857	4	0	1	-.008	4	0	1
91		8	max	270.922	1	0	1	.016	10	0	1	0	10	0	1
92			min	2.931	12	0	1	-15.914	4	0	1	-.01	4	0	1
93		9	max	270.987	1	0	1	.016	10	0	1	0	10	0	1
94			min	2.963	12	0	1	-15.97	4	0	1	-.011	4	0	1
95		10	max	271.052	1	0	1	.016	10	0	1	0	10	0	1
96			min	2.996	12	0	1	-16.026	4	0	1	-.013	4	0	1
97		11	max	271.116	1	0	1	.016	10	0	1	0	10	0	1
98			min	3.028	12	0	1	-16.082	4	0	1	-.014	4	0	1
99		12	max	271.181	1	0	1	.016	10	0	1	0	10	0	1
100			min	3.06	12	0	1	-16.138	4	0	1	-.016	4	0	1
101		13	max	271.246	1	0	1	.016	10	0	1	0	10	0	1
102			min	3.093	12	0	1	-16.194	4	0	1	-.017	4	0	1
103		14	max	271.31	1	0	1	.016	10	0	1	0	10	0	1
104			min	3.125	12	0	1	-16.25	4	0	1	-.018	4	0	1
105		15	max	271.375	1	0	1	.016	10	0	1	0	10	0	1
106			min	3.157	12	0	1	-16.306	4	0	1	-.02	4	0	1
107		16	max	271.44	1	0	1	.016	10	0	1	0	10	0	1
108			min	3.19	12	0	1	-16.362	4	0	1	-.021	4	0	1
109		17	max	271.504	1	0	1	.016	10	0	1	0	10	0	1
110			min	3.222	12	0	1	-16.418	4	0	1	-.023	4	0	1
111		18	max	271.569	1	0	1	.016	10	0	1	0	10	0	1
112			min	3.254	12	0	1	-16.474	4	0	1	-.024	4	0	1
113		19	max	271.634	1	0	1	.016	10	0	1	0	10	0	1
114			min	3.287	12	0	1	-16.53	4	0	1	-.026	4	0	1
115	M6	1	max	632.538	2	.638	6	1.024	4	0	3	0	3	0	1
116			min	-1097.981	3	.142	15	-.272	3	0	5	0	2	0	1
117		2	max	632.663	2	.587	6	.91	4	0	3	0	3	0	15
118			min	-1097.887	3	.13	15	-.272	3	0	5	0	2	0	6
119		3	max	632.789	2	.536	6	.796	4	0	3	0	4	0	15
120			min	-1097.792	3	.118	15	-.272	3	0	5	0	2	0	6
121		4	max	632.915	2	.487	2	.681	4	0	3	0	4	0	15
122			min	-1097.698	3	.106	15	-.272	3	0	5	0	2	0	6
123		5	max	633.041	2	.447	2	.567	4	0	3	0	4	0	15
124			min	-1097.603	3	.094	15	-.272	3	0	5	0	2	0	6
125		6	max	633.167	2	.407	2	.452	4	0	3	0	4	0	15
126			min	-1097.509	3	.082	15	-.272	3	0	5	0	2	0	6
127		7	max	633.293	2	.367	2	.338	4	0	3	0	4	0	15
128			min	-1097.415	3	.067	12	-.272	3	0	5	0	3	0	2
129		8	max	633.419	2	.327	2	.223	4	0	3	0	4	0	15
130			min	-1097.32	3	.047	12	-.272	3	0	5	0	3	0	2
131		9	max	633.545	2	.288	2	.109	4	0	3	0	4	0	15
132			min	-1097.226	3	.027	12	-.272	3	0	5	0	3	0	2
133		10	max	633.67	2	.248	2	.023	9	0	3	0	4	0	15
134			min	-1097.131	3	-.001	3	-.272	3	0	5	0	3	0	2
135		11	max	633.796	2	.208	2	.023	9	0	3	0	4	0	15
136			min	-1097.037	3	-.031	3	-.272	3	0	5	0	3	0	2
137		12	max	633.922	2	.168	2	.023	9	0	3	0	4	0	15
138			min	-1096.943	3	-.061	3	-.272	3	0	5	0	3	0	2
139		13	max	634.048	2	.128	2	.023	9	0	3	0	4	0	12
140			min	-1096.848	3	-.091	3	-.358	5	0	5	0	3	0	2
141		14	max	634.174	2	.088	2	.023	9	0	3	0	4	0	12
142			min	-1096.754	3	-.121	3	-.472	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	634.3	2	.048	2	.023	9	0	3	0	4	0	12
144		min	-1096.659	3	-.151	3	-.587	5	0	5	0	3	0	2
145	16	max	634.426	2	.009	2	.023	9	0	3	0	4	0	12
146		min	-1096.565	3	-.181	3	-.701	5	0	5	0	3	0	2
147	17	max	634.551	2	-.031	2	.023	9	0	3	0	4	0	12
148		min	-1096.471	3	-.211	3	-.816	5	0	5	0	3	0	2
149	18	max	634.677	2	-.062	15	.023	9	0	3	0	4	0	3
150		min	-1096.376	3	-.24	3	-.93	5	0	5	0	3	0	2
151	19	max	634.803	2	-.074	15	.023	9	0	3	0	9	0	3
152		min	-1096.282	3	-.283	4	-1.045	5	0	5	0	3	0	2
153	M7	1	max	601.396	2	1.776	.037	3	0	9	0	4	0	2
154		min	-501.204	3	.424	15	-1.325	4	0	3	0	3	0	3
155	2	max	601.327	2	1.599	4	.037	3	0	9	0	4	0	2
156		min	-501.256	3	.382	15	-1.191	4	0	3	0	3	0	3
157	3	max	601.257	2	1.422	4	.037	3	0	9	0	1	0	2
158		min	-501.308	3	.341	15	-1.057	4	0	3	0	3	0	3
159	4	max	601.188	2	1.245	4	.037	3	0	9	0	1	0	2
160		min	-501.36	3	.299	15	-.924	4	0	3	0	3	0	3
161	5	max	601.119	2	1.068	4	.037	3	0	9	0	1	0	15
162		min	-501.412	3	.258	15	-.79	4	0	3	0	5	0	3
163	6	max	601.049	2	.891	4	.037	3	0	9	0	1	0	15
164		min	-501.464	3	.216	15	-.656	4	0	3	0	5	0	6
165	7	max	600.98	2	.715	4	.037	3	0	9	0	1	0	15
166		min	-501.516	3	.174	15	-.523	4	0	3	0	5	0	6
167	8	max	600.911	2	.538	4	.037	3	0	9	0	1	0	15
168		min	-501.568	3	.133	15	-.389	4	0	3	0	5	-.001	6
169	9	max	600.841	2	.361	4	.037	3	0	9	0	1	0	15
170		min	-501.62	3	.078	12	-.255	4	0	3	0	5	-.001	6
171	10	max	600.772	2	.212	2	.037	3	0	9	0	1	0	15
172		min	-501.672	3	.004	3	-.122	4	0	3	0	5	-.001	6
173	11	max	600.703	2	.074	2	.037	3	0	9	0	1	0	15
174		min	-501.724	3	-.1	3	-.014	1	0	3	0	5	-.001	6
175	12	max	600.633	2	-.033	15	.147	5	0	9	0	1	0	15
176		min	-501.776	3	-.203	3	-.014	1	0	3	0	5	-.001	6
177	13	max	600.564	2	-.075	15	.28	5	0	9	0	1	0	15
178		min	-501.828	3	-.347	6	-.014	1	0	3	0	5	-.001	6
179	14	max	600.495	2	-.117	15	.414	5	0	9	0	1	0	15
180		min	-501.88	3	-.524	6	-.014	1	0	3	0	5	-.001	6
181	15	max	600.425	2	-.158	15	.548	5	0	9	0	1	0	15
182		min	-501.932	3	-.701	6	-.014	1	0	3	0	5	0	6
183	16	max	600.356	2	-.2	15	.681	5	0	9	0	1	0	15
184		min	-501.984	3	-.878	6	-.014	1	0	3	0	5	0	6
185	17	max	600.287	2	-.241	15	.815	5	0	9	0	1	0	15
186		min	-502.036	3	-1.054	6	-.014	1	0	3	0	5	0	6
187	18	max	600.217	2	-.283	15	.949	5	0	9	0	9	0	15
188		min	-502.088	3	-1.231	6	-.014	1	0	3	0	3	0	6
189	19	max	600.148	2	-.324	15	1.082	5	0	9	0	9	0	1
190		min	-502.14	3	-1.408	6	-.014	1	0	3	0	3	0	1
191	M8	1	max	751.859	1	0	.156	9	0	1	0	4	0	1
192		min	-38.243	3	0	1	-15.788	4	0	1	0	3	0	1
193	2	max	751.923	1	0	1	.156	9	0	1	0	9	0	1
194		min	-38.194	3	0	1	-15.844	4	0	1	-.001	4	0	1
195	3	max	751.988	1	0	1	.156	9	0	1	0	9	0	1
196		min	-38.146	3	0	1	-15.9	4	0	1	-.003	4	0	1
197	4	max	752.053	1	0	1	.156	9	0	1	0	9	0	1
198		min	-38.097	3	0	1	-15.956	4	0	1	-.004	4	0	1
199	5	max	752.117	1	0	1	.156	9	0	1	0	9	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-38.049	3	0	1	-16.012	4	0	1	-.006	4	0	1
201		6	max	752.182	1	0	1	.156	9	0	1	0	9	0	1
202			min	-38	3	0	1	-16.068	4	0	1	-.007	4	0	1
203		7	max	752.247	1	0	1	.156	9	0	1	0	9	0	1
204			min	-37.952	3	0	1	-16.124	4	0	1	-.009	4	0	1
205		8	max	752.312	1	0	1	.156	9	0	1	0	9	0	1
206			min	-37.903	3	0	1	-16.18	4	0	1	-.01	4	0	1
207		9	max	752.376	1	0	1	.156	9	0	1	0	9	0	1
208			min	-37.855	3	0	1	-16.237	4	0	1	-.011	4	0	1
209		10	max	752.441	1	0	1	.156	9	0	1	0	9	0	1
210			min	-37.806	3	0	1	-16.293	4	0	1	-.013	4	0	1
211		11	max	752.506	1	0	1	.156	9	0	1	0	9	0	1
212			min	-37.758	3	0	1	-16.349	4	0	1	-.014	4	0	1
213		12	max	752.57	1	0	1	.156	9	0	1	0	9	0	1
214			min	-37.709	3	0	1	-16.405	4	0	1	-.016	4	0	1
215		13	max	752.635	1	0	1	.156	9	0	1	0	9	0	1
216			min	-37.661	3	0	1	-16.461	4	0	1	-.017	4	0	1
217		14	max	752.7	1	0	1	.156	9	0	1	0	9	0	1
218			min	-37.612	3	0	1	-16.517	4	0	1	-.019	4	0	1
219		15	max	752.765	1	0	1	.156	9	0	1	0	9	0	1
220			min	-37.563	3	0	1	-16.573	4	0	1	-.02	4	0	1
221		16	max	752.829	1	0	1	.156	9	0	1	0	9	0	1
222			min	-37.515	3	0	1	-16.629	4	0	1	-.022	4	0	1
223		17	max	752.894	1	0	1	.156	9	0	1	0	9	0	1
224			min	-37.466	3	0	1	-16.685	4	0	1	-.023	4	0	1
225		18	max	752.959	1	0	1	.156	9	0	1	0	9	0	1
226			min	-37.418	3	0	1	-16.741	4	0	1	-.025	4	0	1
227		19	max	753.023	1	0	1	.156	9	0	1	0	9	0	1
228			min	-37.369	3	0	1	-16.797	4	0	1	-.026	4	0	1
229	M10	1	max	203.493	2	.686	4	1.134	5	0	1	0	1	0	1
230			min	-291.773	3	.175	15	-.106	1	-.001	5	0	3	0	1
231		2	max	203.619	2	.635	4	1.02	5	0	1	0	4	0	15
232			min	-291.678	3	.163	15	-.106	1	-.001	5	0	3	0	4
233		3	max	203.745	2	.584	4	.905	5	0	1	0	4	0	15
234			min	-291.584	3	.15	15	-.106	1	-.001	5	0	3	0	4
235		4	max	203.87	2	.532	4	.791	5	0	1	0	4	0	15
236			min	-291.49	3	.138	15	-.106	1	-.001	5	0	3	0	4
237		5	max	203.996	2	.481	4	.676	5	0	1	0	4	0	15
238			min	-291.395	3	.126	15	-.106	1	-.001	5	0	3	0	4
239		6	max	204.122	2	.43	4	.562	5	0	1	0	4	0	15
240			min	-291.301	3	.114	15	-.106	1	-.001	5	0	3	0	4
241		7	max	204.248	2	.379	4	.447	5	0	1	0	4	0	15
242			min	-291.206	3	.102	15	-.106	1	-.001	5	0	3	0	4
243		8	max	204.374	2	.328	4	.333	5	0	1	0	4	0	15
244			min	-291.112	3	.09	15	-.106	1	-.001	5	0	3	0	4
245		9	max	204.5	2	.277	4	.219	5	0	1	0	4	0	15
246			min	-291.018	3	.078	15	-.106	1	-.001	5	0	3	0	4
247		10	max	204.626	2	.226	4	.104	5	0	1	0	5	0	15
248			min	-290.923	3	.062	12	-.106	1	-.001	5	0	3	0	4
249		11	max	204.752	2	.174	4	0	10	0	1	0	5	0	15
250			min	-290.829	3	.042	12	-.106	1	-.001	5	0	3	0	4
251		12	max	204.877	2	.123	4	0	10	0	1	0	5	0	15
252			min	-290.734	3	.022	12	-.139	4	-.001	5	0	3	0	4
253		13	max	205.003	2	.072	4	0	10	0	1	0	5	0	15
254			min	-290.64	3	.001	3	-.254	4	-.001	5	0	3	0	4
255		14	max	205.129	2	.027	5	0	10	0	1	0	5	0	15
256			min	-290.546	3	-.029	3	-.368	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	205.255	2	.008	5	0	10	0	1	0	5	0	15
258		min	-290.451	3	-.059	3	-.483	4	-.001	5	0	3	0	4
259	16	max	205.381	2	-.006	15	0	10	0	1	0	5	0	15
260		min	-290.357	3	-.089	3	-.597	4	-.001	5	0	3	0	4
261	17	max	205.507	2	-.018	15	0	10	0	1	0	5	0	15
262		min	-290.262	3	-.134	6	-.712	4	-.001	5	0	3	0	4
263	18	max	205.633	2	-.03	15	0	10	0	1	0	5	0	12
264		min	-290.168	3	-.185	6	-.826	4	-.001	5	0	3	0	4
265	19	max	205.759	2	-.042	15	0	10	0	1	0	5	0	12
266		min	-290.074	3	-.236	6	-.94	4	-.001	5	0	3	0	4
267	M11	1	max	186.827	2	1.747	.151	1	0	4	0	5	0	2
268		min	-178.778	3	.405	15	-1.263	5	0	10	0	1	0	15
269	2	max	186.758	2	1.57	6	.151	1	0	4	0	3	0	2
270		min	-178.83	3	.363	15	-1.13	5	0	10	0	1	0	3
271	3	max	186.689	2	1.393	6	.151	1	0	4	0	3	0	2
272		min	-178.882	3	.322	15	-.996	5	0	10	0	1	0	3
273	4	max	186.619	2	1.216	6	.151	1	0	4	0	3	0	15
274		min	-178.934	3	.28	15	-.862	5	0	10	0	1	0	4
275	5	max	186.55	2	1.039	6	.151	1	0	4	0	3	0	15
276		min	-178.986	3	.239	15	-.729	5	0	10	0	1	0	4
277	6	max	186.481	2	.862	6	.151	1	0	4	0	3	0	15
278		min	-179.038	3	.197	15	-.595	5	0	10	0	4	0	4
279	7	max	186.411	2	.686	6	.151	1	0	4	0	3	0	15
280		min	-179.09	3	.156	15	-.461	5	0	10	0	4	0	4
281	8	max	186.342	2	.509	6	.151	1	0	4	0	3	0	15
282		min	-179.142	3	.114	15	-.327	5	0	10	0	4	-.001	4
283	9	max	186.273	2	.332	6	.151	1	0	4	0	3	0	15
284		min	-179.194	3	.072	15	-.194	5	0	10	0	4	-.001	4
285	10	max	186.203	2	.157	2	.151	1	0	4	0	3	0	15
286		min	-179.246	3	.031	15	-.06	5	0	10	0	4	-.001	4
287	11	max	186.134	2	.019	2	.151	1	0	4	0	3	0	15
288		min	-179.298	3	-.045	3	-.055	3	0	10	0	4	-.001	4
289	12	max	186.065	2	-.052	15	.244	4	0	4	0	3	0	15
290		min	-179.35	3	-.199	4	-.055	3	0	10	0	4	-.001	4
291	13	max	185.995	2	-.094	15	.378	4	0	4	0	3	0	15
292		min	-179.402	3	-.376	4	-.055	3	0	10	0	4	-.001	4
293	14	max	185.926	2	-.135	15	.511	4	0	4	0	3	0	15
294		min	-179.454	3	-.553	4	-.055	3	0	10	0	4	-.001	4
295	15	max	185.857	2	-.177	15	.645	4	0	4	0	3	0	15
296		min	-179.506	3	-.73	4	-.055	3	0	10	0	5	0	4
297	16	max	185.788	2	-.219	15	.779	4	0	4	0	3	0	15
298		min	-179.558	3	-.906	4	-.055	3	0	10	0	5	0	4
299	17	max	185.718	2	-.26	15	.912	4	0	4	0	3	0	15
300		min	-179.61	3	-1.083	4	-.055	3	0	10	0	10	0	4
301	18	max	185.649	2	-.302	15	1.046	4	0	4	0	3	0	15
302		min	-179.662	3	-1.26	4	-.055	3	0	10	0	10	0	4
303	19	max	185.58	2	-.343	15	1.18	4	0	4	0	4	0	1
304		min	-179.714	3	-1.437	4	-.055	3	0	10	0	10	0	1
305	M12	1	max	270.691	1	0	.781	1	0	1	0	4	0	1
306		min	.62	15	0	1	-14.514	5	0	1	0	3	0	1
307	2	max	270.756	1	0	1	.781	1	0	1	0	1	0	1
308		min	.64	15	0	1	-14.57	5	0	1	-.001	5	0	1
309	3	max	270.82	1	0	1	.781	1	0	1	0	1	0	1
310		min	.659	15	0	1	-14.626	5	0	1	-.003	5	0	1
311	4	max	270.885	1	0	1	.781	1	0	1	0	1	0	1
312		min	.679	15	0	1	-14.683	5	0	1	-.004	5	0	1
313	5	max	270.95	1	0	1	.781	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	.698	15	0	1	-14.739	5	0	1	-.005	5	0	1
315		6	max	271.014	1	0	1	.781	1	0	1	0	1	0	1
316			min	.718	15	0	1	-14.795	5	0	1	-.007	5	0	1
317		7	max	271.079	1	0	1	.781	1	0	1	0	1	0	1
318			min	.737	15	0	1	-14.851	5	0	1	-.008	5	0	1
319		8	max	271.144	1	0	1	.781	1	0	1	0	1	0	1
320			min	.757	15	0	1	-14.907	5	0	1	-.009	5	0	1
321		9	max	271.209	1	0	1	.781	1	0	1	0	1	0	1
322			min	.777	15	0	1	-14.963	5	0	1	-.011	5	0	1
323		10	max	271.273	1	0	1	.781	1	0	1	0	1	0	1
324			min	.796	15	0	1	-15.019	5	0	1	-.012	5	0	1
325		11	max	271.338	1	0	1	.781	1	0	1	0	1	0	1
326			min	.816	15	0	1	-15.075	5	0	1	-.013	5	0	1
327		12	max	271.403	1	0	1	.781	1	0	1	0	1	0	1
328			min	.835	15	0	1	-15.131	5	0	1	-.015	5	0	1
329		13	max	271.467	1	0	1	.781	1	0	1	0	1	0	1
330			min	.855	15	0	1	-15.187	5	0	1	-.016	5	0	1
331		14	max	271.532	1	0	1	.781	1	0	1	0	1	0	1
332			min	.874	15	0	1	-15.243	5	0	1	-.017	5	0	1
333		15	max	271.597	1	0	1	.781	1	0	1	0	1	0	1
334			min	.894	15	0	1	-15.299	5	0	1	-.019	5	0	1
335		16	max	271.661	1	0	1	.781	1	0	1	.001	1	0	1
336			min	.913	15	0	1	-15.356	5	0	1	-.02	5	0	1
337		17	max	271.726	1	0	1	.781	1	0	1	.001	1	0	1
338			min	.933	15	0	1	-15.412	5	0	1	-.021	5	0	1
339		18	max	271.791	1	0	1	.781	1	0	1	.001	1	0	1
340			min	.952	15	0	1	-15.468	5	0	1	-.023	5	0	1
341		19	max	271.856	1	0	1	.781	1	0	1	.001	1	0	1
342			min	.972	15	0	1	-15.524	5	0	1	-.024	5	0	1
343	M1	1	max	76.151	1	342.466	3	.243	10	0	2	.036	1	0	2
344			min	6.027	10	-222.182	2	-18.198	1	0	3	0	10	0	3
345		2	max	76.291	1	342.285	3	.243	10	0	2	.032	1	.048	2
346			min	6.144	10	-222.424	2	-18.198	1	0	3	0	10	-.075	3
347		3	max	90.115	3	4.664	4	.244	10	0	10	.028	1	.096	2
348			min	-16.15	10	-24.984	2	-18.139	1	0	1	0	10	-.147	3
349		4	max	90.22	3	4.384	14	.244	10	0	10	.024	1	.101	2
350			min	-16.034	10	-25.226	2	-18.139	1	0	1	0	10	-.145	3
351		5	max	90.325	3	4.147	14	.244	10	0	10	.02	1	.107	2
352			min	-15.917	10	-25.468	2	-18.139	1	0	1	0	10	-.142	3
353		6	max	90.43	3	3.909	14	.244	10	0	10	.016	1	.112	2
354			min	-15.801	10	-25.709	2	-18.139	1	0	1	0	10	-.139	3
355		7	max	90.534	3	3.672	14	.244	10	0	10	.012	1	.118	2
356			min	-15.685	10	-25.951	2	-18.139	1	0	1	0	10	-.136	3
357		8	max	90.639	3	3.434	14	.244	10	0	10	.008	1	.124	2
358			min	-15.568	10	-26.193	2	-18.139	1	0	1	0	10	-.133	3
359		9	max	90.744	3	3.196	14	.244	10	0	10	.004	1	.129	2
360			min	-15.452	10	-26.435	2	-18.139	1	0	1	0	10	-.13	3
361		10	max	90.848	3	2.959	14	.244	10	0	10	.002	3	.135	2
362			min	-15.336	10	-26.677	2	-18.139	1	0	1	0	10	-.127	3
363		11	max	90.953	3	2.721	14	.244	10	0	10	0	3	.141	2
364			min	-15.219	10	-26.919	2	-18.139	1	0	1	-.004	1	-.124	3
365		12	max	91.058	3	2.484	14	.244	10	0	10	0	10	.147	2
366			min	-15.103	10	-27.16	2	-18.139	1	0	1	-.008	1	-.121	3
367		13	max	91.163	3	2.246	14	.244	10	0	10	0	10	.153	2
368			min	-14.986	10	-27.402	2	-18.139	1	0	1	-.012	1	-.118	3
369		14	max	91.267	3	2.038	9	.244	10	0	10	0	10	.159	2
370			min	-14.87	10	-27.644	2	-18.139	1	0	1	-.016	1	-.115	3









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	34.627	4	32.173	2	10.441	4	0	3	0	5	.257	3
486			min	-.243	10	-52.739	3	-3.331	2	0	2	-.032	1	-.169	2
487		16	max	30.262	4	46.052	3	19.526	1	0	3	.005	5	.259	3
488			min	-.243	10	-31.394	2	-.268	10	0	2	-.028	1	-.169	2
489		17	max	25.896	4	144.844	3	38.399	1	0	3	.01	5	.217	3
490			min	-.243	10	-94.962	2	1.83	10	0	2	-.015	1	-.141	2
491		18	max	21.531	4	243.636	3	57.273	1	0	3	.018	4	.13	3
492			min	-.243	10	-158.53	2	3.929	10	0	2	-.003	10	-.085	2
493		19	max	18.231	1	342.428	3	76.146	1	0	3	.036	1	0	2
494			min	-.243	10	-222.097	2	6.027	10	0	2	0	10	0	3
495	M16	1	max	30.325	5	328.149	2	6.745	5	0	3	.036	1	0	2
496			min	-18.847	1	-163.202	3	-76.06	1	0	2	-.025	5	0	3
497		2	max	25.96	5	233.932	2	7.921	5	0	3	.006	1	.062	3
498			min	-18.847	1	-116.953	3	-57.186	1	0	2	-.021	5	-.125	2
499		3	max	21.595	5	139.714	2	9.096	5	0	3	0	3	.104	3
500			min	-18.847	1	-70.704	3	-38.313	1	0	2	-.021	4	-.208	2
501		4	max	17.23	5	45.497	2	10.271	5	0	3	-.002	12	.125	3
502			min	-18.847	1	-24.455	3	-19.439	1	0	2	-.028	1	-.249	2
503		5	max	12.865	5	21.795	3	11.446	5	0	3	-.003	12	.126	3
504			min	-18.847	1	-48.72	2	-4.024	3	0	2	-.032	1	-.248	2
505		6	max	8.5	5	68.044	3	18.308	1	0	3	-.002	15	.106	3
506			min	-18.847	1	-142.938	2	-2.919	3	0	2	-.029	1	-.206	2
507		7	max	4.134	5	114.293	3	37.181	1	0	3	.003	5	.065	3
508			min	-18.847	1	-237.155	2	-1.813	3	0	2	-.016	1	-.121	2
509		8	max	2.294	3	160.542	3	56.055	1	0	3	.01	4	.005	2
510			min	-18.847	1	-331.373	2	-.708	3	0	2	-.008	3	0	15
511		9	max	2.294	3	206.791	3	74.928	1	0	3	.034	1	.173	2
512			min	-18.847	1	-425.59	2	.398	3	0	2	-.008	3	-.077	3
513		10	max	17.872	5	-7.245	15	93.802	1	0	14	.071	1	.383	2
514			min	-18.847	1	-519.808	2	-2.83	3	0	2	-.007	3	-.18	3
515		11	max	13.506	5	425.59	2	4.099	5	0	2	.033	1	.173	2
516			min	-18.816	1	-206.791	3	-74.835	1	0	3	-.01	5	-.077	3
517		12	max	9.141	5	331.373	2	5.274	5	0	2	.006	2	.005	2
518			min	-18.816	1	-160.542	3	-55.961	1	0	3	-.008	5	0	15
519		13	max	4.776	5	237.155	2	6.449	5	0	2	0	10	.065	3
520			min	-18.816	1	-114.293	3	-37.088	1	0	3	-.016	1	-.121	2
521		14	max	.411	5	142.938	2	7.624	5	0	2	-.001	12	.106	3
522			min	-18.816	1	-68.044	3	-18.214	1	0	3	-.029	1	-.206	2
523		15	max	.261	10	48.72	2	9.486	4	0	2	.002	5	.126	3
524			min	-18.816	1	-21.795	3	-3.307	2	0	3	-.032	1	-.248	2
525		16	max	.261	10	24.455	3	19.533	1	0	2	.006	5	.125	3
526			min	-18.816	1	-45.497	2	-.255	10	0	3	-.028	1	-.249	2
527		17	max	.261	10	70.704	3	38.406	1	0	2	.011	5	.104	3
528			min	-18.816	1	-139.714	2	1.844	10	0	3	-.015	1	-.208	2
529		18	max	.261	10	116.953	3	57.28	1	0	2	.018	4	.062	3
530			min	-21.643	4	-233.932	2	3.845	12	0	3	-.003	10	-.125	2
531		19	max	.261	10	163.202	3	76.153	1	0	2	.036	1	0	2
532			min	-26.008	4	-328.149	2	4.582	12	0	3	0	10	0	5
533	M15	1	max	0	1	.835	3	.136	3	0	1	0	1	0	1
534			min	-112.072	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.742	3	.136	3	0	1	0	1	0	1
536			min	-112.143	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.649	3	.136	3	0	1	0	1	0	1
538			min	-112.213	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.556	3	.136	3	0	1	0	1	0	1
540			min	-112.284	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.464	3	.136	3	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-112.354	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.371	3	.136	3	0	1	0	1	0	1
544			min	-112.425	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.278	3	.136	3	0	1	0	3	0	1
546			min	-112.495	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.185	3	.136	3	0	1	0	3	0	1
548			min	-112.566	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.093	3	.136	3	0	1	0	3	0	1
550			min	-112.636	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.136	3	0	1	0	3	0	1
552			min	-112.707	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.136	3	0	1	0	3	0	1
554			min	-112.777	3	-.093	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.136	3	0	1	0	3	0	1
556			min	-112.848	3	-.185	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.136	3	0	1	0	3	0	1
558			min	-112.918	3	-.278	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.136	3	0	1	0	3	0	1
560			min	-112.989	3	-.371	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.136	3	0	1	0	3	0	1
562			min	-113.059	3	-.464	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.136	3	0	1	0	3	0	1
564			min	-113.13	3	-.556	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.136	3	0	1	0	3	0	1
566			min	-113.2	3	-.649	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.136	3	0	1	0	3	0	1
568			min	-113.271	3	-.742	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.136	3	0	1	0	3	0	1
570			min	-113.341	3	-.835	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.14	4	.322	4	0	3	0	3	0	1
572			min	-173.795	4	0	2	-.056	3	0	1	0	4	0	1
573		2	max	0	2	1.902	4	.289	4	0	3	0	3	0	2
574			min	-173.778	4	0	2	-.056	3	0	1	0	4	0	4
575		3	max	0	2	1.664	4	.257	4	0	3	0	3	0	2
576			min	-173.762	4	0	2	-.056	3	0	1	0	4	-.001	4
577		4	max	0	2	1.426	4	.225	4	0	3	0	3	0	2
578			min	-173.745	4	0	2	-.056	3	0	1	0	1	-.002	4
579		5	max	0	2	1.189	4	.192	4	0	3	0	3	0	2
580			min	-173.728	4	0	2	-.056	3	0	1	0	1	-.002	4
581		6	max	0	2	.951	4	.16	4	0	3	0	3	0	2
582			min	-173.712	4	0	2	-.056	3	0	1	0	1	-.002	4
583		7	max	0	2	.713	4	.127	4	0	3	0	3	0	2
584			min	-173.695	4	0	2	-.056	3	0	1	0	1	-.002	4
585		8	max	0	2	.475	4	.095	4	0	3	0	5	0	2
586			min	-173.679	4	0	2	-.056	3	0	1	0	1	-.003	4
587		9	max	0	2	.238	4	.063	4	0	3	0	5	0	2
588			min	-173.662	4	0	2	-.056	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.038	1	0	3	0	5	0	2
590			min	-173.645	4	0	1	-.056	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.038	1	0	3	0	5	0	2
592			min	-173.629	4	-.238	4	-.056	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.038	1	0	3	0	5	0	2
594			min	-173.612	4	-.475	4	-.056	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.038	1	0	3	0	5	0	2
596			min	-173.595	4	-.713	4	-.07	5	0	1	0	3	-.002	4
597		14	max	0	2	0	2	.038	1	0	3	0	5	0	2
598			min	-173.579	4	-.951	4	-.102	5	0	1	0	3	-.002	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.088	1	0	2	.038	1	0	3	0	5	0	2
600		min	-173.562	4	-1.189	4	-.135	5	0	1	0	3	-.002	4
601	16	max	.182	1	0	2	.038	1	0	3	0	1	0	2
602		min	-173.546	4	-1.426	4	-.167	5	0	1	0	3	-.002	4
603	17	max	.276	1	0	2	.038	1	0	3	0	1	0	2
604		min	-173.529	4	-1.664	4	-.199	5	0	1	0	3	-.001	4
605	18	max	.37	1	0	2	.038	1	0	3	0	1	0	2
606		min	-173.587	5	-1.902	4	-.232	5	0	1	0	5	0	4
607	19	max	.464	1	0	2	.038	1	0	3	0	1	0	1
608		min	-173.647	5	-2.14	4	-.264	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.009	2	.003	1	8.937e-4	5	NC	3	NC	1
2			min	-.004	3	-.009	3	-.01	5	-2.913e-4	1	4188.08	2	NC	1
3		2	max	.002	2	.009	2	.003	1	9.145e-4	5	NC	3	NC	1
4			min	-.003	3	-.009	3	-.01	5	-2.78e-4	1	4570.535	2	NC	1
5		3	max	.002	2	.008	2	.002	1	9.353e-4	5	NC	3	NC	1
6			min	-.003	3	-.009	3	-.009	5	-2.648e-4	1	5025.344	2	NC	1
7		4	max	.002	2	.007	2	.002	1	9.561e-4	5	NC	1	NC	1
8			min	-.003	3	-.008	3	-.009	5	-2.515e-4	1	5569.952	2	NC	1
9		5	max	.002	2	.006	2	.002	1	9.769e-4	5	NC	1	NC	1
10			min	-.003	3	-.008	3	-.009	5	-2.382e-4	1	6227.603	2	NC	1
11		6	max	.001	2	.006	2	.002	1	9.977e-4	5	NC	1	NC	1
12			min	-.003	3	-.008	3	-.008	5	-2.25e-4	1	7029.782	2	NC	1
13		7	max	.001	2	.005	2	.002	1	1.019e-3	5	NC	1	NC	1
14			min	-.002	3	-.007	3	-.008	5	-2.117e-4	1	8019.939	2	NC	1
15		8	max	.001	2	.004	2	.001	1	1.039e-3	5	NC	1	NC	1
16			min	-.002	3	-.007	3	-.008	5	-1.984e-4	1	9259.388	2	NC	1
17		9	max	.001	2	.004	2	.001	1	1.06e-3	5	NC	1	NC	1
18			min	-.002	3	-.006	3	-.007	5	-1.852e-4	1	NC	1	NC	1
19		10	max	0	2	.003	2	.001	1	1.081e-3	5	NC	1	NC	1
20		min	-.002	3	-.006	3	-.007	5	-1.719e-4	1	NC	1	NC	1	
21	11	max	0	2	.003	2	0	1	1.102e-3	5	NC	1	NC	1	
22		min	-.002	3	-.005	3	-.006	5	-1.586e-4	1	NC	1	NC	1	
23	12	max	0	2	.002	2	0	1	1.122e-3	5	NC	1	NC	1	
24		min	-.001	3	-.005	3	-.006	5	-1.454e-4	1	NC	1	NC	1	
25	13	max	0	2	.002	2	0	1	1.143e-3	5	NC	1	NC	1	
26		min	-.001	3	-.004	3	-.005	5	-1.321e-4	1	NC	1	NC	1	
27	14	max	0	2	.001	2	0	1	1.164e-3	5	NC	1	NC	1	
28		min	0	3	-.003	3	-.004	5	-1.188e-4	1	NC	1	NC	1	
29	15	max	0	2	0	2	0	1	1.185e-3	5	NC	1	NC	1	
30		min	0	3	-.003	3	-.003	5	-1.056e-4	1	NC	1	NC	1	
31	16	max	0	2	0	2	0	1	1.206e-3	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.003	5	-9.231e-5	1	NC	1	NC	1	
33	17	max	0	2	0	2	0	1	1.226e-3	5	NC	1	NC	1	
34		min	0	3	-.001	3	-.002	5	-7.904e-5	1	NC	1	NC	1	
35	18	max	0	2	0	2	0	1	1.247e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-6.578e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.268e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-5.251e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.485e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.976e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	3.308e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-6.012e-4	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	4.131e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.049e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.009	5	4.954e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	9	-6.085e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.012	5	5.777e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-6.122e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.015	4	6.6e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-6.158e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.018	4	7.423e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-6.195e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.021	4	8.246e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-6.232e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.024	4	9.07e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-6.268e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.027	4	9.893e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-6.305e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.029	4	1.072e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-6.341e-4	5	NC	1	NC	1
61		12	max	.001	3	.003	2	.032	4	1.154e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-6.378e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.034	4	1.236e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-6.414e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.037	4	1.319e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-6.451e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.039	4	1.401e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-6.487e-4	5	9076.236	2	NC	1
69		16	max	.002	3	.006	2	.041	4	1.483e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-6.524e-4	5	7682.131	2	NC	1
71		17	max	.002	3	.007	2	.043	4	1.565e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-6.56e-4	5	6605.083	2	NC	1
73		18	max	.002	3	.008	2	.045	4	1.648e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-6.597e-4	5	5763.382	2	NC	1
75		19	max	.002	3	.009	2	.048	4	1.73e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-6.634e-4	5	5099.653	2	NC	1
77	M4	1	max	.001	1	.011	2	0	10	3.258e-3	5	NC	1	NC	1
78			min	0	12	-.009	3	-.05	4	-2.195e-4	1	NC	1	385.249	4
79		2	max	.001	1	.01	2	0	10	3.258e-3	5	NC	1	NC	1
80			min	0	12	-.009	3	-.046	4	-2.195e-4	1	NC	1	419.924	4
81		3	max	.001	1	.01	2	0	10	3.258e-3	5	NC	1	NC	1
82			min	0	12	-.008	3	-.042	4	-2.195e-4	1	NC	1	461.189	4
83		4	max	.001	1	.009	2	0	10	3.258e-3	5	NC	1	NC	1
84			min	0	12	-.008	3	-.038	4	-2.195e-4	1	NC	1	510.778	4
85		5	max	.001	1	.008	2	0	10	3.258e-3	5	NC	1	NC	1
86			min	0	12	-.007	3	-.034	4	-2.195e-4	1	NC	1	571.057	4
87		6	max	0	1	.008	2	0	10	3.258e-3	5	NC	1	NC	1
88			min	0	12	-.007	3	-.03	4	-2.195e-4	1	NC	1	645.311	4
89		7	max	0	1	.007	2	0	10	3.258e-3	5	NC	1	NC	1
90			min	0	12	-.006	3	-.026	4	-2.195e-4	1	NC	1	738.223	4
91		8	max	0	1	.007	2	0	10	3.258e-3	5	NC	1	NC	1
92			min	0	12	-.006	3	-.023	4	-2.195e-4	1	NC	1	856.647	4
93		9	max	0	1	.006	2	0	10	3.258e-3	5	NC	1	NC	1
94			min	0	12	-.005	3	-.019	4	-2.195e-4	1	NC	1	1010.95	4
95		10	max	0	1	.005	2	0	10	3.258e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.016	4	-2.195e-4	1	NC	1	1217.426	4
97		11	max	0	1	.005	2	0	10	3.258e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.013	4	-2.195e-4	1	NC	1	1502.913	4
99		12	max	0	1	.004	2	0	10	3.258e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.004	3	-.01	4	-2.195e-4	1	NC	1	1914.218	4
101		max	0	1	.004	2	0	10	3.258e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.008	4	-2.195e-4	1	NC	1	2539.135	4
103		max	0	1	.003	2	0	10	3.258e-3	5	NC	1	NC	1
104		min	0	12	-.003	3	-.005	4	-2.195e-4	1	NC	1	3558.747	4
105		max	0	1	.002	2	0	10	3.258e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.004	4	-2.195e-4	1	NC	1	5398.452	4
107		max	0	1	.002	2	0	10	3.258e-3	5	NC	1	NC	1
108		min	0	12	-.002	3	-.002	4	-2.195e-4	1	NC	1	9267.483	4
109		max	0	1	.001	2	0	10	3.258e-3	5	NC	1	NC	1
110		min	0	12	-.001	3	0	4	-2.195e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	3.258e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-2.195e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	3.258e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-2.195e-4	1	NC	1	NC	1
115	M6	max	.006	2	.03	2	0	9	9.495e-4	4	NC	3	NC	1
116		min	-.011	3	-.028	3	-.01	5	-8.418e-8	2	1310.583	2	6193.628	3
117		max	.006	2	.028	2	0	9	9.705e-4	4	NC	3	NC	1
118		min	-.01	3	-.027	3	-.01	5	-9.632e-7	1	1403.083	2	6571.734	3
119		max	.005	2	.026	2	0	9	9.915e-4	4	NC	3	NC	1
120		min	-.009	3	-.025	3	-.009	5	-2.419e-6	1	1509.175	2	7021.708	3
121		max	.005	2	.024	2	0	9	1.012e-3	4	NC	3	NC	1
122		min	-.009	3	-.024	3	-.009	5	-3.875e-6	1	1631.585	2	7558.815	3
123		max	.005	2	.022	2	0	9	1.033e-3	4	NC	3	NC	1
124		min	-.008	3	-.022	3	-.009	5	-5.33e-6	1	1773.83	2	8203.336	3
125		max	.004	2	.02	2	0	9	1.054e-3	4	NC	3	NC	1
126		min	-.008	3	-.021	3	-.009	5	-6.786e-6	1	1940.512	2	8982.593	3
127		max	.004	2	.018	2	0	9	1.075e-3	4	NC	3	NC	1
128		min	-.007	3	-.019	3	-.008	5	-8.242e-6	1	2137.776	2	9934.042	3
129		max	.004	2	.017	2	0	9	1.096e-3	4	NC	3	NC	1
130		min	-.007	3	-.018	3	-.008	5	-9.697e-6	1	2374.003	2	NC	1
131		max	.003	2	.015	2	0	9	1.117e-3	4	NC	3	NC	1
132		min	-.006	3	-.016	3	-.008	5	-1.115e-5	1	2660.931	2	NC	1
133		max	.003	2	.013	2	0	9	1.138e-3	4	NC	3	NC	1
134		min	-.005	3	-.015	3	-.007	5	-1.261e-5	1	3015.515	2	NC	1
135		max	.003	2	.011	2	0	9	1.159e-3	4	NC	3	NC	1
136		min	-.005	3	-.013	3	-.006	5	-1.406e-5	1	3463.181	2	NC	1
137		max	.002	2	.01	2	0	9	1.18e-3	4	NC	3	NC	1
138		min	-.004	3	-.011	3	-.006	5	-1.552e-5	1	4043.88	2	NC	1
139		max	.002	2	.008	2	0	9	1.201e-3	4	NC	3	NC	1
140		min	-.004	3	-.01	3	-.005	5	-1.698e-5	1	4824.172	2	NC	1
141		max	.002	2	.007	2	0	9	1.222e-3	4	NC	1	NC	1
142		min	-.003	3	-.008	3	-.004	5	-1.843e-5	1	5923.842	2	NC	1
143		max	.001	2	.005	2	0	9	1.243e-3	4	NC	1	NC	1
144		min	-.002	3	-.007	3	-.004	5	-1.989e-5	1	7582.402	2	NC	1
145		max	.001	2	.004	2	0	9	1.264e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.003	5	-2.134e-5	1	NC	1	NC	1
147		max	0	2	.002	2	0	1	1.285e-3	4	NC	1	NC	1
148		min	-.001	3	-.003	3	-.002	5	-2.28e-5	1	NC	1	NC	1
149		max	0	2	.001	2	0	1	1.306e-3	4	NC	1	NC	1
150		min	0	3	-.002	3	0	5	-2.425e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.327e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-2.571e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.21e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.251e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.003	4	1.12e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-6.187e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.006	4	1.029e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-6.122e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.01	4	9.377e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-6.058e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.013	4	8.468e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-5.993e-4	4	8968.057	2	NC	1
163		6	max	.002	3	.006	2	.016	4	2.429e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	-5.929e-4	4	7184.156	2	NC	1
165		7	max	.002	3	.008	2	.019	4	4.944e-5	3	NC	1	NC	1
166			min	-.002	2	-.011	3	0	1	-5.864e-4	4	5963.445	2	NC	1
167		8	max	.002	3	.009	2	.022	4	7.459e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-5.8e-4	4	5068.095	2	NC	1
169		9	max	.003	3	.011	2	.025	4	9.973e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	0	1	-5.735e-4	4	4379.616	2	NC	1
171		10	max	.003	3	.012	2	.028	4	1.249e-4	3	NC	3	NC	1
172			min	-.003	2	-.016	3	0	1	-5.671e-4	4	3832.322	2	NC	1
173		11	max	.003	3	.014	2	.03	4	1.5e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	-5.606e-4	4	3386.746	2	NC	1
175		12	max	.003	3	.015	2	.033	4	1.752e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	0	9	-5.542e-4	4	3017.606	2	NC	1
177		13	max	.004	3	.017	2	.035	4	2.003e-4	3	NC	3	NC	1
178			min	-.005	2	-.019	3	0	9	-5.477e-4	4	2707.821	2	NC	1
179		14	max	.004	3	.019	2	.038	4	2.255e-4	3	NC	3	NC	1
180			min	-.005	2	-.021	3	0	9	-5.413e-4	4	2445.332	2	NC	1
181		15	max	.004	3	.021	2	.04	4	2.506e-4	3	NC	3	NC	1
182			min	-.005	2	-.022	3	0	9	-5.348e-4	4	2221.299	2	NC	1
183		16	max	.005	3	.023	2	.042	4	2.758e-4	3	NC	3	NC	1
184			min	-.006	2	-.023	3	0	9	-5.283e-4	4	2029.046	2	NC	1
185		17	max	.005	3	.025	2	.045	4	3.009e-4	3	NC	3	NC	1
186			min	-.006	2	-.024	3	0	9	-5.219e-4	4	1863.406	2	NC	1
187		18	max	.005	3	.027	2	.047	4	3.261e-4	3	NC	3	NC	1
188			min	-.006	2	-.024	3	0	9	-5.154e-4	4	1720.302	2	NC	1
189		19	max	.006	3	.029	2	.049	4	3.512e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	9	-5.09e-4	4	1596.473	2	NC	1
191	M8	1	max	.004	1	.034	2	0	9	3.107e-3	4	NC	1	NC	1
192			min	0	3	-.028	3	-.051	4	-2.595e-4	3	NC	1	378.907	4
193		2	max	.003	1	.032	2	0	9	3.107e-3	4	NC	1	NC	1
194			min	0	3	-.027	3	-.047	4	-2.595e-4	3	NC	1	413.013	4
195		3	max	.003	1	.03	2	0	9	3.107e-3	4	NC	1	NC	1
196			min	0	3	-.025	3	-.043	4	-2.595e-4	3	NC	1	453.599	4
197		4	max	.003	1	.029	2	0	9	3.107e-3	4	NC	1	NC	1
198			min	0	3	-.023	3	-.038	4	-2.595e-4	3	NC	1	502.375	4
199		5	max	.003	1	.027	2	0	9	3.107e-3	4	NC	1	NC	1
200			min	0	3	-.022	3	-.034	4	-2.595e-4	3	NC	1	561.664	4
201		6	max	.003	1	.025	2	0	9	3.107e-3	4	NC	1	NC	1
202			min	0	3	-.02	3	-.03	4	-2.595e-4	3	NC	1	634.7	4
203		7	max	.002	1	.023	2	0	9	3.107e-3	4	NC	1	NC	1
204			min	0	3	-.019	3	-.027	4	-2.595e-4	3	NC	1	726.088	4
205		8	max	.002	1	.021	2	0	9	3.107e-3	4	NC	1	NC	1
206			min	0	3	-.017	3	-.023	4	-2.595e-4	3	NC	1	842.57	4
207		9	max	.002	1	.019	2	0	9	3.107e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.019	4	-2.595e-4	3	NC	1	994.343	4
209		10	max	.002	1	.017	2	0	9	3.107e-3	4	NC	1	NC	1
210			min	0	3	-.014	3	-.016	4	-2.595e-4	3	NC	1	1197.435	4
211		11	max	.002	1	.015	2	0	9	3.107e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.013	4	-2.595e-4	3	NC	1	1478.244	4
213		12	max	.001	1	.013	2	0	9	3.107e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.387e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.856e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.008	4	2.932e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-6.273e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.01	4	4.779e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-6.69e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.013	4	8.013e-7	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-7.106e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.016	5	9.e-7	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-7.523e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.018	5	9.987e-7	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-7.94e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.021	5	1.097e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-8.357e-4	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.023	5	1.196e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-8.773e-4	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.025	5	1.295e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-9.19e-4	4	NC	1	NC	1
289		12	max	.001	3	.003	2	.028	5	1.393e-6	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.003	3	-9.607e-4	4	NC	1	NC	1
291		13	max	.001	3	.003	2	.03	5	1.492e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-1.002e-3	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.032	5	1.591e-6	10	NC	1	NC	1
294			min	-.002	2	-.008	3	-.003	3	-1.044e-3	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.034	5	1.69e-6	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.003	3	-1.086e-3	4	9088.772	2	NC	1
297		16	max	.002	3	.006	2	.037	5	1.788e-6	10	NC	1	NC	1
298			min	-.002	2	-.008	3	-.003	3	-1.127e-3	4	7691.709	2	NC	1
299		17	max	.002	3	.007	2	.039	5	1.887e-6	10	NC	1	NC	1
300			min	-.002	2	-.008	3	-.003	3	-1.169e-3	4	6612.603	2	NC	1
301		18	max	.002	3	.008	2	.041	5	1.986e-6	10	NC	1	NC	1
302			min	-.002	2	-.008	3	-.003	1	-1.211e-3	4	5769.443	2	NC	1
303		19	max	.002	3	.009	2	.043	5	2.084e-6	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.003	1	-1.252e-3	4	5104.66	2	NC	1
305	M12	1	max	.001	1	.011	2	.003	1	3.771e-3	4	NC	1	NC	2
306			min	0	15	-.009	3	-.047	5	-2.916e-6	10	NC	1	411.392	5
307		2	max	.001	1	.01	2	.002	1	3.771e-3	4	NC	1	NC	2
308			min	0	15	-.009	3	-.043	5	-2.916e-6	10	NC	1	448.409	5
309		3	max	.001	1	.01	2	.002	1	3.771e-3	4	NC	1	NC	2
310			min	0	15	-.008	3	-.039	5	-2.916e-6	10	NC	1	492.461	5
311		4	max	.001	1	.009	2	.002	1	3.771e-3	4	NC	1	NC	1
312			min	0	15	-.008	3	-.035	5	-2.916e-6	10	NC	1	545.398	5
313		5	max	.001	1	.008	2	.002	1	3.771e-3	4	NC	1	NC	1
314			min	0	15	-.007	3	-.032	5	-2.916e-6	10	NC	1	609.745	5
315		6	max	0	1	.008	2	.001	1	3.771e-3	4	NC	1	NC	1
316			min	0	15	-.007	3	-.028	5	-2.916e-6	10	NC	1	689.009	5
317		7	max	0	1	.007	2	.001	1	3.771e-3	4	NC	1	NC	1
318			min	0	15	-.006	3	-.025	5	-2.916e-6	10	NC	1	788.188	5
319		8	max	0	1	.007	2	.001	1	3.771e-3	4	NC	1	NC	1
320			min	0	15	-.006	3	-.021	5	-2.916e-6	10	NC	1	914.597	5
321		9	max	0	1	.006	2	0	1	3.771e-3	4	NC	1	NC	1
322			min	0	15	-.005	3	-.018	5	-2.916e-6	10	NC	1	1079.301	5
323		10	max	0	1	.005	2	0	1	3.771e-3	4	NC	1	NC	1
324			min	0	15	-.005	3	-.015	5	-2.916e-6	10	NC	1	1299.692	5
325		11	max	0	1	.005	2	0	1	3.771e-3	4	NC	1	NC	1
326			min	0	15	-.004	3	-.012	5	-2.916e-6	10	NC	1	1604.413	5
327		12	max	0	1	.004	2	0	1	3.771e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	15	-.004	3	-.009	5	-2.916e-6	10	NC	1	2043.419	5
329		13	max	0	1	.004	2	0	1	3.771e-3	4	NC	1	NC	1
330			min	0	15	-.003	3	-.007	5	-2.916e-6	10	NC	1	2710.413	5
331		14	max	0	1	.003	2	0	1	3.771e-3	4	NC	1	NC	1
332			min	0	15	-.003	3	-.005	5	-2.916e-6	10	NC	1	3798.655	5
333		15	max	0	1	.002	2	0	1	3.771e-3	4	NC	1	NC	1
334			min	0	15	-.002	3	-.003	5	-2.916e-6	10	NC	1	5762.15	5
335		16	max	0	1	.002	2	0	1	3.771e-3	4	NC	1	NC	1
336			min	0	15	-.002	3	-.002	5	-2.916e-6	10	NC	1	9891.436	5
337		17	max	0	1	.001	2	0	1	3.771e-3	4	NC	1	NC	1
338			min	0	15	-.001	3	0	5	-2.916e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.771e-3	4	NC	1	NC	1
340			min	0	15	0	3	0	5	-2.916e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.771e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-2.916e-6	10	NC	1	NC	1
343	M1	1	max	.009	3	.025	3	.006	5	5.928e-3	2	NC	1	NC	1
344			min	-.009	2	-.021	2	0	9	-8.687e-3	3	NC	1	NC	1
345		2	max	.008	3	.015	3	.008	5	2.923e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.002	1	-4.279e-3	3	4707.979	3	NC	1
347		3	max	.008	3	.005	3	.01	5	2.875e-4	5	NC	4	NC	1
348			min	-.009	2	-.004	2	-.003	1	-1.467e-4	1	2438.764	3	NC	1
349		4	max	.008	3	.003	2	.012	5	2.891e-4	5	NC	4	NC	1
350			min	-.009	2	-.003	3	-.003	1	-1.227e-4	1	1737.732	3	7500.775	5
351		5	max	.008	3	.01	2	.015	5	2.907e-4	5	NC	4	NC	1
352			min	-.009	2	-.009	3	-.003	1	-9.88e-5	1	1403.461	3	5321.885	5
353		6	max	.008	3	.015	2	.017	5	2.923e-4	5	NC	4	NC	1
354			min	-.009	2	-.015	3	-.003	1	-7.487e-5	1	1216.919	3	4061.403	5
355		7	max	.008	3	.019	2	.02	5	2.939e-4	5	NC	4	NC	1
356			min	-.009	2	-.019	3	-.003	1	-5.172e-5	9	1106.057	2	3251.904	5
357		8	max	.008	3	.023	2	.023	5	2.955e-4	5	NC	4	NC	1
358			min	-.009	2	-.022	3	-.002	1	-3.36e-5	9	1019.529	2	2695.1	5
359		9	max	.008	3	.025	2	.026	5	2.971e-4	5	NC	4	NC	1
360			min	-.009	2	-.023	3	-.001	9	-1.549e-5	9	967.761	2	2293.067	5
361		10	max	.008	3	.026	2	.029	5	3.023e-4	4	NC	4	NC	1
362			min	-.009	2	-.024	3	0	9	1.695e-7	10	943.043	2	1975.889	4
363		11	max	.008	3	.025	2	.033	4	3.1e-4	4	NC	4	NC	1
364			min	-.009	2	-.023	3	0	9	-1.965e-7	10	942.536	2	1733.508	4
365		12	max	.008	3	.024	2	.036	4	3.178e-4	4	NC	4	NC	1
366			min	-.009	2	-.021	3	0	10	-5.626e-7	10	967.225	2	1546.105	4
367		13	max	.008	3	.021	2	.039	4	3.255e-4	4	NC	4	NC	1
368			min	-.009	2	-.018	3	0	10	-9.286e-7	10	1022.495	2	1398.801	4
369		14	max	.008	3	.016	2	.042	4	3.333e-4	4	NC	4	NC	1
370			min	-.009	2	-.014	3	0	10	-1.295e-6	10	1120.886	2	1281.643	4
371		15	max	.008	3	.01	2	.045	4	3.41e-4	4	NC	4	NC	1
372			min	-.009	2	-.009	3	0	10	-1.661e-6	10	1290.039	2	1187.784	4
373		16	max	.008	3	.003	2	.048	4	5.234e-4	4	NC	4	NC	1
374			min	-.009	2	-.003	3	0	10	-1.931e-6	10	1597.846	2	1112.399	4
375		17	max	.008	3	.005	3	.05	4	4.86e-3	4	NC	4	NC	1
376			min	-.009	2	-.006	2	0	10	-9.393e-6	9	2263.728	2	1052.113	4
377		18	max	.008	3	.012	3	.052	4	4.26e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	10	-2.253e-3	3	4387.504	2	1004.268	4
379		19	max	.008	3	.021	3	.054	4	8.59e-3	2	NC	1	NC	1
380			min	-.009	2	-.028	2	0	9	-4.619e-3	3	NC	1	968.165	4
381	M5	1	max	.025	3	.078	3	.006	5	1.696e-5	4	NC	1	NC	1
382			min	-.028	2	-.065	2	0	9	3.72e-8	1	NC	1	NC	1
383		2	max	.025	3	.046	3	.008	5	1.434e-4	5	NC	4	NC	1
384			min	-.028	2	-.038	2	0	9	-1.572e-5	9	1507.405	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.025	3	.016	3	.01	5	2.679e-4	5	NC	5	NC	1
386		min	-.028	2	-.012	2	0	9	-3.125e-5	9	781.279	3	NC	1
387	4	max	.025	3	.011	2	.012	5	2.784e-4	5	NC	5	NC	1
388		min	-.028	2	-.009	3	0	9	-2.977e-5	9	557.431	3	NC	1
389	5	max	.025	3	.031	2	.015	5	2.889e-4	5	NC	5	NC	1
390		min	-.028	2	-.03	3	0	9	-2.829e-5	9	450.822	3	9368.834	3
391	6	max	.025	3	.048	2	.018	5	2.994e-4	5	NC	5	NC	1
392		min	-.028	2	-.046	3	0	9	-2.681e-5	9	391.444	3	8457.521	3
393	7	max	.025	3	.061	2	.021	5	3.099e-4	5	NC	5	NC	1
394		min	-.028	2	-.059	3	0	9	-2.533e-5	9	348.751	2	8039.285	3
395	8	max	.025	3	.072	2	.024	5	3.204e-4	5	NC	5	NC	1
396		min	-.028	2	-.067	3	0	9	-2.385e-5	9	321.379	2	7949.458	3
397	9	max	.025	3	.078	2	.028	5	3.309e-4	5	NC	5	NC	1
398		min	-.028	2	-.072	3	0	9	-2.237e-5	9	304.993	2	8126.19	3
399	10	max	.025	3	.081	2	.031	5	3.415e-4	5	NC	5	NC	1
400		min	-.028	2	-.073	3	0	9	-2.089e-5	9	297.152	2	8562.681	3
401	11	max	.025	3	.08	2	.034	5	3.52e-4	5	NC	5	NC	1
402		min	-.028	2	-.071	3	0	9	-1.941e-5	9	296.957	2	9295.422	3
403	12	max	.025	3	.075	2	.038	4	3.625e-4	5	NC	5	NC	1
404		min	-.028	2	-.065	3	0	9	-1.793e-5	9	304.716	2	NC	1
405	13	max	.024	3	.065	2	.041	4	3.73e-4	5	NC	5	NC	1
406		min	-.028	2	-.056	3	0	9	-1.645e-5	9	322.123	2	NC	1
407	14	max	.024	3	.051	2	.044	4	3.835e-4	5	NC	5	NC	1
408		min	-.028	2	-.043	3	0	9	-1.497e-5	9	353.135	2	NC	1
409	15	max	.024	3	.033	2	.046	4	3.94e-4	5	NC	5	NC	1
410		min	-.028	2	-.027	3	0	9	-1.349e-5	9	406.47	2	NC	1
411	16	max	.024	3	.009	2	.049	4	5.77e-4	4	NC	5	NC	1
412		min	-.028	2	-.008	3	0	9	-1.312e-5	9	503.555	2	NC	1
413	17	max	.024	3	.014	3	.051	4	4.86e-3	4	NC	5	NC	1
414		min	-.028	2	-.02	2	0	9	-3.921e-5	9	713.745	2	NC	1
415	18	max	.024	3	.038	3	.053	4	2.495e-3	4	NC	4	NC	1
416		min	-.028	2	-.053	2	0	9	-2.006e-5	9	1383.807	2	NC	1
417	19	max	.024	3	.063	3	.054	4	5.446e-6	5	NC	1	NC	1
418		min	-.028	2	-.088	2	0	9	-1.426e-6	3	NC	1	NC	1
419	M9	1	max	.009	.024	3	.005	5	8.704e-3	3	NC	1	NC	1
420		min	-.009	2	-.021	2	-.001	9	-5.928e-3	2	NC	1	NC	1
421	2	max	.009	3	.014	3	.005	4	4.282e-3	3	NC	4	NC	1
422		min	-.009	2	-.012	2	0	10	-2.923e-3	2	4710.139	3	NC	1
423	3	max	.009	3	.005	3	.005	4	9.605e-5	1	NC	4	NC	1
424		min	-.009	2	-.004	2	0	10	-5.836e-5	3	2439.901	3	NC	1
425	4	max	.009	3	.003	2	.006	4	7.438e-5	1	NC	4	NC	1
426		min	-.009	2	-.003	3	-.001	3	-6.348e-5	3	1738.517	3	NC	1
427	5	max	.008	3	.01	2	.007	4	5.271e-5	1	NC	4	NC	1
428		min	-.009	2	-.01	3	-.002	3	-6.859e-5	3	1404.045	3	9126.892	3
429	6	max	.008	3	.015	2	.009	4	3.104e-5	1	NC	4	NC	1
430		min	-.009	2	-.015	3	-.003	3	-7.371e-5	3	1217.371	3	7938.936	3
431	7	max	.008	3	.019	2	.011	4	1.598e-5	11	NC	4	NC	1
432		min	-.009	2	-.019	3	-.004	3	-7.882e-5	3	1106.342	2	7253.483	3
433	8	max	.008	3	.023	2	.014	4	5.025e-6	11	NC	4	NC	1
434		min	-.009	2	-.022	3	-.005	3	-8.394e-5	3	1019.801	2	5377.354	4
435	9	max	.008	3	.025	2	.017	4	1.186e-6	5	NC	4	NC	1
436		min	-.009	2	-.024	3	-.005	3	-8.905e-5	3	968.029	2	4011.334	4
437	10	max	.008	3	.026	2	.02	5	8.75e-6	5	NC	4	NC	1
438		min	-.009	2	-.024	3	-.005	3	-9.416e-5	3	943.312	2	3137.471	4
439	11	max	.008	3	.025	2	.024	5	1.631e-5	5	NC	4	NC	1
440		min	-.009	2	-.023	3	-.005	3	-9.928e-5	3	942.812	2	2543.264	4
441	12	max	.008	3	.024	2	.027	5	2.388e-5	5	NC	4	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-0.009	2	-.021	3	-.005	3	-1.044e-4	3	967.516	2	2113.735	5
443		13	max	.008	3	.021	2	.031	5	3.144e-5	5	NC	4	NC	1
444			min	-0.009	2	-.018	3	-.004	3	-1.207e-4	1	1022.808	2	1795.326	5
445		14	max	.008	3	.016	2	.035	5	3.901e-5	5	NC	4	NC	1
446			min	-0.009	2	-.014	3	-.004	3	-1.423e-4	1	1121.234	2	1557.845	5
447		15	max	.008	3	.01	2	.039	5	4.657e-5	5	NC	4	NC	1
448			min	-0.009	2	-.009	3	-.003	3	-1.64e-4	1	1290.443	2	1376.42	5
449		16	max	.008	3	.003	2	.043	5	2.411e-4	5	NC	4	NC	1
450			min	-0.009	2	-.003	3	-.003	1	-1.803e-4	1	1598.341	2	1235.258	5
451		17	max	.008	3	.005	3	.047	5	4.889e-3	4	NC	4	NC	1
452			min	-0.009	2	-.006	2	-.003	1	-6.921e-5	9	2264.38	2	1123.819	5
453		18	max	.008	3	.012	3	.05	5	2.417e-3	5	NC	4	NC	1
454			min	-0.009	2	-.017	2	-.002	1	-4.26e-3	2	4388.727	2	1031.353	4
455		19	max	.008	3	.021	3	.054	4	4.616e-3	3	NC	1	NC	1
456			min	-0.009	2	-.028	2	0	9	-8.591e-3	2	NC	1	954.361	4
457	M13	1	max	.001	9	.024	3	.009	3	3.839e-3	3	NC	1	NC	1
458			min	-0.005	5	-.021	2	-.009	2	-3.29e-3	2	NC	1	NC	1
459		2	max	.001	9	.069	3	.007	3	4.753e-3	3	NC	4	NC	1
460			min	-0.005	5	-.051	2	-.008	2	-4.08e-3	2	2138.174	3	NC	1
461		3	max	.001	9	.107	3	.008	9	5.667e-3	3	NC	4	NC	2
462			min	-0.005	5	-.077	2	-.007	2	-4.871e-3	2	1160.617	3	9077.61	1
463		4	max	0	9	.133	3	.011	9	6.582e-3	3	NC	5	NC	2
464			min	-0.005	5	-.096	2	-.007	2	-5.662e-3	2	883.822	3	6594.261	1
465		5	max	0	9	.145	3	.013	9	7.496e-3	3	NC	5	NC	2
466			min	-0.005	5	-.105	2	-.009	2	-6.452e-3	2	798.415	3	6257.03	1
467		6	max	0	9	.142	3	.013	3	8.41e-3	3	NC	5	NC	2
468			min	-0.005	5	-.104	2	-.013	2	-7.243e-3	2	815.946	3	7735.987	1
469		7	max	0	9	.128	3	.016	3	9.324e-3	3	NC	4	NC	1
470			min	-0.006	5	-.096	2	-.017	2	-8.034e-3	2	928.352	3	NC	1
471		8	max	0	9	.107	3	.019	3	1.024e-2	3	NC	4	NC	1
472			min	-0.006	5	-.084	2	-.022	2	-8.825e-3	2	1163.675	3	7206.093	2
473		9	max	0	9	.087	3	.022	3	1.115e-2	3	NC	4	NC	1
474			min	-0.006	5	-.071	2	-.026	2	-9.615e-3	2	1539.126	3	5497.748	2
475		10	max	0	9	.078	3	.025	3	1.207e-2	3	NC	4	NC	4
476			min	-0.006	5	-.065	2	-.028	2	-1.041e-2	2	1813.155	3	4983.909	2
477		11	max	0	9	.087	3	.027	3	1.116e-2	3	NC	4	NC	1
478			min	-0.006	5	-.071	2	-.026	2	-9.615e-3	2	1539.125	3	5177.917	3
479		12	max	0	9	.107	3	.027	3	1.024e-2	3	NC	4	NC	1
480			min	-0.006	5	-.084	2	-.022	2	-8.825e-3	2	1163.673	3	5079.545	3
481		13	max	0	9	.128	3	.026	3	9.332e-3	3	NC	4	NC	1
482			min	-0.006	5	-.096	2	-.017	2	-8.034e-3	2	928.351	3	5360.234	3
483		14	max	0	9	.142	3	.024	3	8.42e-3	3	NC	5	NC	2
484			min	-0.006	5	-.104	2	-.013	2	-7.243e-3	2	815.945	3	6079.24	3
485		15	max	0	9	.145	3	.021	3	7.508e-3	3	NC	5	NC	2
486			min	-0.006	5	-.105	2	-.009	2	-6.453e-3	2	798.414	3	6260.521	1
487		16	max	0	9	.134	3	.018	3	6.596e-3	3	NC	5	NC	2
488			min	-0.006	5	-.096	2	-.007	2	-5.662e-3	2	883.821	3	6603.931	1
489		17	max	0	9	.108	3	.014	3	5.684e-3	3	NC	4	NC	2
490			min	-0.006	5	-.077	2	-.007	2	-4.871e-3	2	1160.616	3	9100.991	1
491		18	max	0	9	.07	3	.011	3	4.773e-3	3	NC	4	NC	1
492			min	-0.006	5	-.051	2	-.008	2	-4.081e-3	2	2138.172	3	NC	1
493		19	max	0	9	.025	3	.009	3	3.861e-3	3	NC	1	NC	1
494			min	-0.006	5	-.021	2	-.009	2	-3.29e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.021	3	.008	3	4.195e-3	2	NC	1	NC	1
496			min	-.054	4	-.028	2	-.009	2	-3.14e-3	3	NC	1	NC	1
497		2	max	0	9	.045	3	.011	3	5.207e-3	2	NC	4	NC	1
498			min	-.054	4	-.072	2	-.008	2	-3.853e-3	3	2162.003	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	0	9	.065	3	.014	3	6.219e-3	2	NC	4	NC	2
500		min	-.054	4	-.11	2	-.007	2	-4.566e-3	3	1170.537	2	9114.508	1
501	4	max	0	9	.08	3	.017	3	7.232e-3	2	NC	5	NC	2
502		min	-.054	4	-.136	2	-.007	2	-5.279e-3	3	887.367	2	6621.846	1
503	5	max	0	9	.088	3	.02	3	8.244e-3	2	NC	5	NC	2
504		min	-.054	4	-.148	2	-.009	2	-5.993e-3	3	795.921	2	6288.237	1
505	6	max	0	9	.089	3	.023	3	9.257e-3	2	NC	5	NC	2
506		min	-.054	4	-.147	2	-.013	2	-6.706e-3	3	804.402	2	6678.701	3
507	7	max	0	9	.084	3	.024	3	1.027e-2	2	NC	5	NC	1
508		min	-.054	4	-.134	2	-.017	2	-7.419e-3	3	899.273	2	5973.177	3
509	8	max	0	9	.076	3	.025	3	1.128e-2	2	NC	4	NC	1
510		min	-.054	4	-.115	2	-.022	2	-8.132e-3	3	1096.264	2	5661.608	3
511	9	max	0	9	.067	3	.025	3	1.229e-2	2	NC	4	NC	1
512		min	-.054	4	-.096	2	-.026	2	-8.845e-3	3	1394.056	2	5522.028	2
513	10	max	0	9	.063	3	.024	3	1.331e-2	2	NC	4	NC	4
514		min	-.054	4	-.088	2	-.028	2	-9.559e-3	3	1598.774	2	5004.549	2
515	11	max	0	9	.067	3	.023	3	1.229e-2	2	NC	4	NC	1
516		min	-.054	4	-.096	2	-.026	2	-8.843e-3	3	1394.056	2	5522.035	2
517	12	max	0	9	.076	3	.021	3	1.128e-2	2	NC	4	NC	1
518		min	-.054	4	-.115	2	-.022	2	-8.128e-3	3	1096.264	2	7243.991	2
519	13	max	0	9	.084	3	.02	3	1.027e-2	2	NC	5	NC	1
520		min	-.054	4	-.134	2	-.017	2	-7.412e-3	3	899.273	2	8404.51	3
521	14	max	0	9	.089	3	.018	3	9.257e-3	2	NC	5	NC	2
522		min	-.054	4	-.147	2	-.013	2	-6.697e-3	3	804.402	2	7799.7	1
523	15	max	0	9	.088	3	.015	3	8.245e-3	2	NC	5	NC	2
524		min	-.054	4	-.148	2	-.009	2	-5.981e-3	3	795.921	2	6299.886	1
525	16	max	0	9	.08	3	.013	3	7.233e-3	2	NC	5	NC	2
526		min	-.054	4	-.136	2	-.007	2	-5.266e-3	3	887.367	2	6640.159	1
527	17	max	0	9	.065	3	.011	3	6.221e-3	2	NC	4	NC	2
528		min	-.054	4	-.11	2	-.007	2	-4.55e-3	3	1170.537	2	9151.375	1
529	18	max	0	9	.045	3	.009	3	5.208e-3	2	NC	4	NC	1
530		min	-.054	4	-.072	2	-.008	2	-3.835e-3	3	2162.003	2	NC	1
531	19	max	0	9	.021	3	.008	3	4.196e-3	2	NC	1	NC	1
532		min	-.054	4	-.028	2	-.009	2	-3.119e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	4.042e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.042e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.004	4	8.124e-4	3	NC	1	NC	1
536		min	0	4	-.002	1	0	3	-6.137e-4	5	NC	1	NC	1
537	3	max	0	3	.001	5	.009	4	1.221e-3	3	NC	1	NC	1
538		min	0	4	-.004	1	-.003	3	-8.467e-4	2	NC	1	6889.502	4
539	4	max	0	3	.002	5	.014	4	1.629e-3	3	NC	1	NC	9
540		min	-.001	4	-.006	1	-.007	3	-1.246e-3	2	NC	1	4371.287	4
541	5	max	0	3	.003	5	.019	4	2.037e-3	3	NC	3	NC	9
542		min	-.002	4	-.007	1	-.012	3	-1.645e-3	2	7832.259	1	3236.002	4
543	6	max	0	3	.003	5	.024	4	2.445e-3	3	NC	4	NC	9
544		min	-.002	4	-.009	1	-.017	3	-2.043e-3	2	6591.674	1	2467.774	3
545	7	max	0	3	.004	5	.027	4	2.853e-3	3	NC	5	8131.026	9
546		min	-.003	4	-.01	1	-.022	3	-2.442e-3	2	5845.625	1	1932.194	3
547	8	max	0	3	.004	5	.03	4	3.261e-3	3	NC	5	6779.71	9
548		min	-.003	4	-.011	1	-.027	3	-2.841e-3	2	5397.884	1	1595.048	3
549	9	max	0	3	.005	5	.031	4	3.67e-3	3	NC	5	5887.876	9
550		min	-.004	4	-.011	1	-.032	3	-3.24e-3	2	5156.883	1	1374.227	3
551	10	max	0	3	.005	5	.031	4	4.078e-3	3	NC	5	5297.569	9
552		min	-.004	4	-.011	1	-.035	3	-3.639e-3	2	5080.642	1	1228.431	3
553	11	max	.001	3	.005	5	.029	2	4.486e-3	3	NC	5	4925.374	9
554		min	-.004	4	-.011	9	-.038	3	-4.038e-3	2	5156.883	1	1135.961	3
555	12	max	.001	3	.005	5	.03	2	4.894e-3	3	NC	5	4730.611	9



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.005	4	-.01	9	-.038	3	-4.437e-3	2	5397.884	1	1086.067	3
557	13	max	.001	3	.006	5	.029	2	5.302e-3	3	NC	5	5305.532	15
558		min	-.005	4	-.01	9	-.037	3	-4.836e-3	2	5845.625	1	1075.676	3
559	14	max	.001	3	.006	5	.025	2	5.71e-3	3	NC	4	7039.111	15
560		min	-.006	4	-.009	9	-.034	3	-5.235e-3	2	6591.674	1	1109.544	3
561	15	max	.001	3	.006	5	.021	1	6.119e-3	3	NC	3	NC	15
562		min	-.006	4	-.007	9	-.028	3	-5.634e-3	2	7832.259	1	1204.964	3
563	16	max	.002	3	.006	5	.014	1	6.527e-3	3	NC	1	NC	5
564		min	-.007	4	-.006	9	-.019	3	-6.033e-3	2	NC	1	1408.825	3
565	17	max	.002	3	.006	5	.006	1	6.935e-3	3	NC	1	NC	4
566		min	-.007	4	-.004	9	-.006	3	-6.431e-3	2	NC	1	1868.178	3
567	18	max	.002	3	.006	5	.01	3	7.343e-3	3	NC	1	NC	4
568		min	-.008	4	-.003	9	-.012	2	-6.83e-3	2	NC	1	3326.847	3
569	19	max	.002	3	.007	2	.031	3	7.751e-3	3	NC	1	NC	1
570		min	-.008	4	-.001	9	-.03	2	-7.229e-3	2	NC	1	NC	1
571	M16A	1	max	0	.002	2	.009	3	2.206e-3	3	NC	1	NC	1
572		min	-.003	4	-.004	4	-.009	2	-2.242e-3	2	NC	1	NC	1
573	2	max	0	2	0	2	.002	3	2.124e-3	3	NC	1	NC	1
574		min	-.003	4	-.008	4	-.004	2	-2.14e-3	2	NC	1	9285.847	3
575	3	max	0	2	-.002	10	.003	1	2.041e-3	3	NC	1	NC	4
576		min	-.003	4	-.011	4	-.006	5	-2.039e-3	2	7608.933	4	5259.305	3
577	4	max	0	2	-.004	12	.006	1	1.959e-3	3	NC	1	NC	9
578		min	-.002	4	-.015	4	-.01	5	-1.937e-3	2	5220.172	4	4004.762	3
579	5	max	0	2	-.005	12	.008	1	1.877e-3	3	NC	3	NC	9
580		min	-.002	4	-.018	4	-.014	5	-1.836e-3	2	4073.354	4	3463.339	3
581	6	max	0	2	-.005	12	.009	1	1.794e-3	3	NC	12	NC	9
582		min	-.002	4	-.021	4	-.019	5	-1.734e-3	2	3428.158	4	3229.926	3
583	7	max	0	2	-.006	12	.01	1	1.712e-3	3	NC	12	NC	9
584		min	-.002	4	-.023	4	-.024	5	-1.633e-3	2	3040.157	4	2772.464	5
585	8	max	0	2	-.006	12	.01	1	1.629e-3	3	NC	12	NC	9
586		min	-.002	4	-.024	4	-.027	5	-1.531e-3	2	2807.299	4	2368.329	5
587	9	max	0	2	-.007	12	.009	1	1.547e-3	3	NC	12	NC	9
588		min	-.002	4	-.025	4	-.03	5	-1.43e-3	2	2681.961	4	2138.78	5
589	10	max	0	2	-.007	12	.008	1	1.465e-3	3	NC	12	NC	9
590		min	-.001	4	-.025	4	-.032	5	-1.328e-3	2	2642.31	4	2026.509	5
591	11	max	0	2	-.006	12	.007	1	1.382e-3	3	NC	12	NC	9
592		min	-.001	4	-.025	4	-.032	5	-1.227e-3	2	2681.961	4	2007.538	5
593	12	max	0	2	-.006	12	.006	1	1.3e-3	3	NC	12	NC	9
594		min	-.001	4	-.024	4	-.031	5	-1.125e-3	2	2807.299	4	2078.663	5
595	13	max	0	2	-.006	12	.005	1	1.217e-3	3	NC	12	NC	2
596		min	0	4	-.022	4	-.028	5	-1.024e-3	2	3040.157	4	2256.224	5
597	14	max	0	2	-.005	12	.003	1	1.135e-3	3	NC	12	NC	1
598		min	0	4	-.019	4	-.025	5	-9.222e-4	2	3428.158	4	2585.365	5
599	15	max	0	2	-.004	12	.002	1	1.053e-3	3	NC	3	NC	1
600		min	0	4	-.016	4	-.02	5	-8.207e-4	2	4073.354	4	3170.489	5
601	16	max	0	2	-.003	12	.001	9	9.703e-4	3	NC	1	NC	1
602		min	0	4	-.013	4	-.015	5	-7.192e-4	2	5220.172	4	4272.892	5
603	17	max	0	2	-.002	12	0	9	8.879e-4	3	NC	1	NC	1
604		min	0	4	-.009	4	-.009	5	-6.177e-4	2	7608.933	4	6711.782	5
605	18	max	0	2	-.001	12	0	3	9.05e-4	4	NC	1	NC	1
606		min	0	4	-.004	4	-.004	5	-5.161e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	9.673e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.146e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
<b>Concrete breakout</b>	<b>1465</b>	<b>7233</b>	<b>0.20</b>	<b>Pass (Governs)</b>	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
<b>T Concrete breakout x+</b>	<b>999</b>	<b>4043</b>	<b>0.25</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>999</b>	<b>11720</b>	<b>0.09</b>	<b>Pass (Governs)</b>	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.