

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

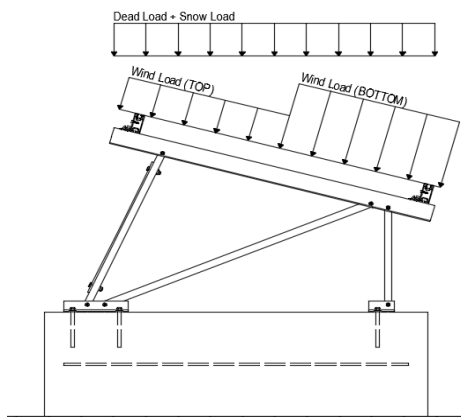
1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	150 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 35.33 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

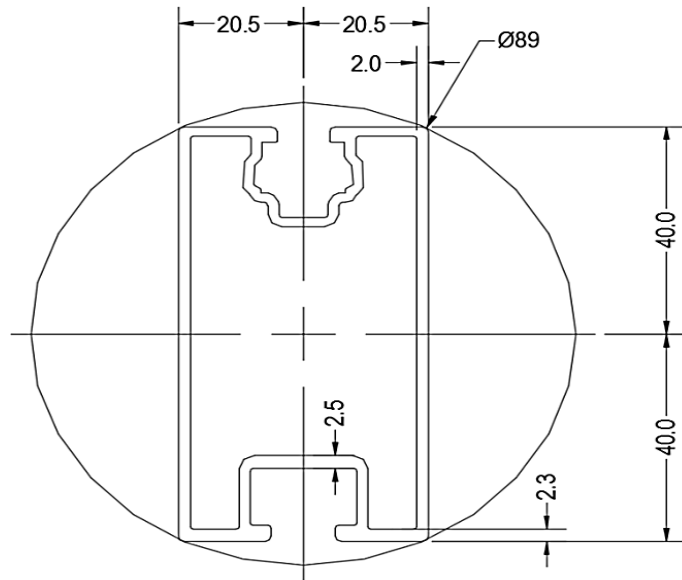
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	51 in
ΦF_{ty} STRONG-AXIS =	30.04 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.358 k-ft
M_z =	0.066 k-ft
$M_{y \text{ allowable}}$ =	1.866 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<u>27%</u>



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.88 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.447 k-ft
M_z =	0.000 k-ft
P_n =	0.251 k
$M_{y \text{ allowable}}$ =	1.466 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<u>32%</u>



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.971 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.314 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.756 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	11%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.172 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

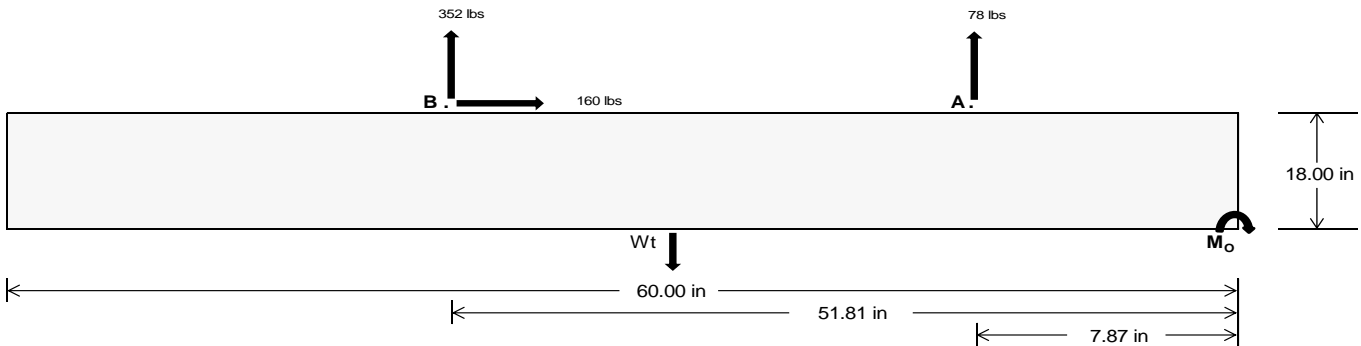
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>342.47</u>	<u>1529.04</u>	k
Compressive Load =	<u>1262.94</u>	<u>1029.77</u>	k
Lateral Load =	<u>22.02</u>	<u>691.95</u>	k
Moment (Weak Axis) =	<u>0.04</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21725.1$ in-lbs
Resisting Force Required = 724.17 lbs
S.F. = 1.67
Weight Required = 1206.95 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 159.65 lbs
Friction = 0.4
Weight Required = 399.12 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 159.65 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

		Ballast Width			
		20 in	21 in	22 in	23 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$		1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	409 lbs	409 lbs	409 lbs	409 lbs	474 lbs	474 lbs	474 lbs	474 lbs	631 lbs	631 lbs	631 lbs	631 lbs	-156 lbs	-156 lbs	-156 lbs	-156 lbs
F_B	289 lbs	289 lbs	289 lbs	289 lbs	415 lbs	415 lbs	415 lbs	415 lbs	505 lbs	505 lbs	505 lbs	505 lbs	-704 lbs	-704 lbs	-704 lbs	-704 lbs
F_V	25 lbs	25 lbs	25 lbs	25 lbs	281 lbs	281 lbs	281 lbs	281 lbs	228 lbs	228 lbs	228 lbs	228 lbs	-319 lbs	-319 lbs	-319 lbs	-319 lbs
P_{total}	2511 lbs	2601 lbs	2692 lbs	2783 lbs	2701 lbs	2792 lbs	2882 lbs	2973 lbs	2948 lbs	3039 lbs	3130 lbs	3220 lbs	228 lbs	282 lbs	337 lbs	391 lbs
M	266 lbs-ft	266 lbs-ft	266 lbs-ft	266 lbs-ft	540 lbs-ft	540 lbs-ft	540 lbs-ft	540 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft	586 lbs-ft	514 lbs-ft	514 lbs-ft	514 lbs-ft	514 lbs-ft
e	0.11 ft	0.10 ft	0.10 ft	0.10 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.26 ft	1.82 ft	1.53 ft	1.31 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	263.0 psf	260.8 psf	258.9 psf	257.1 psf	246.4 psf	245.0 psf	243.8 psf	242.7 psf	269.5 psf	267.0 psf	264.7 psf	262.7 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	339.6 psf	333.8 psf	328.5 psf	323.7 psf	401.9 psf	393.1 psf	385.1 psf	377.8 psf	438.2 psf	427.7 psf	418.1 psf	409.4 psf	372.3 psf	158.3 psf	125.7 psf	114.7 psf

Maximum Bearing Pressure = 438 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

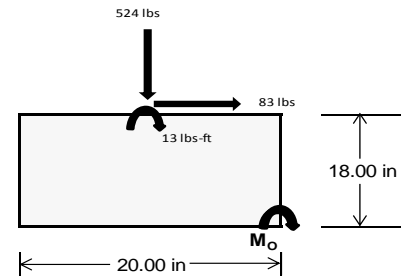
Overturning Check

$M_o = 299.4 \text{ ft-lbs}$
 Resisting Force Required = 359.27 lbs
 S.F. = 1.67
 Weight Required = 598.78 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	111 lbs	74 lbs	55 lbs	240 lbs	524 lbs	197 lbs	73 lbs	-22 lbs	19 lbs
F_v	13 lbs	110 lbs	13 lbs	9 lbs	83 lbs	10 lbs	13 lbs	110 lbs	13 lbs
P_{total}	2355 lbs	2318 lbs	2299 lbs	2376 lbs	2660 lbs	2333 lbs	729 lbs	634 lbs	675 lbs
M	35 lbs-ft	184 lbs-ft	36 lbs-ft	26 lbs-ft	138 lbs-ft	28 lbs-ft	36 lbs-ft	184 lbs-ft	36 lbs-ft
e	0.01 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.29 ft	0.05 ft
$L/6$	0.28 ft	1.51 ft	1.64 ft	1.65 ft	1.56 ft	1.64 ft	1.57 ft	1.09 ft	1.56 ft
f_{min}	267.4 sqft	198.8 sqft	260.4 sqft	274.1 sqft	259.8 sqft	267.7 sqft	71.8 sqft	-3.2 sqft	65.6 sqft
f_{max}	297.7 psf	357.4 psf	291.4 psf	296.1 psf	378.7 psf	292.3 psf	103.0 psf	155.4 psf	96.5 psf



Maximum Bearing Pressure = 379 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.704 k
Allowable Uplift =	1.214 k
Utilization =	<u>58%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.066 k
Allowable Uplift =	1.116 k
Utilization =	<u>95%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.971 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Diagonal Strut

Maximum Axial Load =	0.314 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.085 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.172 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.055 in
	<u>0.055 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.427$$

$$106.368$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.427$$

$$115.584$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.0 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.866 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.41 \\
 &20.702 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.41 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.466 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	244.264	1	.081	2	.149	1	0	10	0	4	0	15
30			min	-356.152	3	.014	15	-.467	5	0	4	0	3	0	6
31		16	max	244.37	1	.048	2	.149	1	0	10	0	4	0	15
32			min	-356.073	3	-.005	3	-.563	5	0	4	0	3	0	6
33		17	max	244.477	1	.016	2	.149	1	0	10	0	14	0	15
34			min	-355.993	3	-.029	3	-.66	5	0	4	0	3	0	6
35		18	max	244.583	1	-.015	15	.149	1	0	10	0	1	0	15
36			min	-355.913	3	-.058	4	-.756	5	0	4	0	3	0	6
37		19	max	244.69	1	-.025	15	.149	1	0	10	0	1	0	15
38			min	-355.833	3	-.1	4	-.852	5	0	4	0	3	0	6
39	M3	1	max	95.759	2	1.795	6	0	10	0	5	0	4	0	6
40			min	-85.676	3	.421	15	-1.36	4	0	1	0	10	0	15
41		2	max	95.692	2	1.618	6	0	10	0	5	0	1	0	6
42			min	-85.727	3	.379	15	-1.227	4	0	1	0	10	0	15
43		3	max	95.624	2	1.44	6	0	10	0	5	0	1	0	2
44			min	-85.778	3	.337	15	-1.093	4	0	1	0	10	0	3
45		4	max	95.556	2	1.262	6	0	10	0	5	0	1	0	15
46			min	-85.829	3	.295	15	-.96	4	0	1	0	5	0	4
47		5	max	95.488	2	1.085	6	0	10	0	5	0	1	0	15
48			min	-85.88	3	.254	15	-.826	4	0	1	0	5	0	4
49		6	max	95.42	2	.907	6	0	10	0	5	0	1	0	15
50			min	-85.931	3	.212	15	-.692	4	0	1	0	5	0	4
51		7	max	95.352	2	.729	6	0	10	0	5	0	1	0	15
52			min	-85.982	3	.17	15	-.559	4	0	1	0	5	0	4
53		8	max	95.284	2	.552	6	0	10	0	5	0	1	0	15
54			min	-86.033	3	.128	15	-.425	4	0	1	0	5	0	4
55		9	max	95.217	2	.374	6	0	10	0	5	0	1	0	15
56			min	-86.084	3	.087	15	-.292	4	0	1	0	5	-.001	4
57		10	max	95.149	2	.196	6	0	10	0	5	0	1	0	15
58			min	-86.134	3	.045	15	-.166	1	0	1	0	5	-.001	4
59		11	max	95.081	2	.036	2	.011	5	0	5	0	1	0	15
60			min	-86.185	3	-.003	3	-.166	1	0	1	0	5	-.001	4
61		12	max	95.013	2	-.039	15	.145	5	0	5	0	1	0	15
62			min	-86.236	3	-.159	4	-.166	1	0	1	0	5	-.001	4
63		13	max	94.945	2	-.08	15	.279	5	0	5	0	1	0	15
64			min	-86.287	3	-.337	4	-.166	1	0	1	0	5	-.001	4
65		14	max	94.877	2	-.122	15	.412	5	0	5	0	1	0	15
66			min	-86.338	3	-.514	4	-.166	1	0	1	0	5	-.001	4
67		15	max	94.809	2	-.164	15	.546	5	0	5	0	3	0	15
68			min	-86.389	3	-.692	4	-.166	1	0	1	0	5	0	4
69		16	max	94.742	2	-.206	15	.68	5	0	5	0	10	0	15
70			min	-86.44	3	-.87	4	-.166	1	0	1	0	4	0	4
71		17	max	94.674	2	-.247	15	.813	5	0	5	0	10	0	15
72			min	-86.491	3	-1.047	4	-.166	1	0	1	0	4	0	4
73		18	max	94.606	2	-.289	15	.947	5	0	5	0	10	0	15
74			min	-86.542	3	-1.225	4	-.166	1	0	1	0	4	0	4
75		19	max	94.538	2	-.331	15	1.08	5	0	5	0	5	0	1
76			min	-86.592	3	-1.403	4	-.166	1	0	1	0	1	0	1
77	M4	1	max	335.041	1	0	1	.001	10	0	1	0	5	0	1
78			min	-74.757	3	0	1	-15.737	4	0	1	0	2	0	1
79		2	max	335.106	1	0	1	.001	10	0	1	0	10	0	1
80			min	-74.708	3	0	1	-15.793	4	0	1	-.001	4	0	1
81		3	max	335.17	1	0	1	.001	10	0	1	0	10	0	1
82			min	-74.66	3	0	1	-15.849	4	0	1	-.003	4	0	1
83		4	max	335.235	1	0	1	.001	10	0	1	0	10	0	1
84			min	-74.611	3	0	1	-15.905	4	0	1	-.004	4	0	1
85		5	max	335.3	1	0	1	.001	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-74.563	3	0	1	-15.961	4	0	1	-.006	4	0	1
87		6	max	335.364	1	0	1	.001	10	0	1	0	10	0	1
88			min	-74.514	3	0	1	-16.017	4	0	1	-.007	4	0	1
89		7	max	335.429	1	0	1	.001	10	0	1	0	10	0	1
90			min	-74.466	3	0	1	-16.073	4	0	1	-.009	4	0	1
91		8	max	335.494	1	0	1	.001	10	0	1	0	10	0	1
92			min	-74.417	3	0	1	-16.129	4	0	1	-.01	4	0	1
93		9	max	335.559	1	0	1	.001	10	0	1	0	10	0	1
94			min	-74.369	3	0	1	-16.185	4	0	1	-.011	4	0	1
95		10	max	335.623	1	0	1	.001	10	0	1	0	10	0	1
96			min	-74.32	3	0	1	-16.241	4	0	1	-.013	4	0	1
97		11	max	335.688	1	0	1	.001	10	0	1	0	10	0	1
98			min	-74.272	3	0	1	-16.298	4	0	1	-.014	4	0	1
99		12	max	335.753	1	0	1	.001	10	0	1	0	10	0	1
100			min	-74.223	3	0	1	-16.354	4	0	1	-.016	4	0	1
101		13	max	335.817	1	0	1	.001	10	0	1	0	10	0	1
102			min	-74.175	3	0	1	-16.41	4	0	1	-.017	4	0	1
103		14	max	335.882	1	0	1	.001	10	0	1	0	10	0	1
104			min	-74.126	3	0	1	-16.466	4	0	1	-.019	4	0	1
105		15	max	335.947	1	0	1	.001	10	0	1	0	10	0	1
106			min	-74.077	3	0	1	-16.522	4	0	1	-.02	4	0	1
107		16	max	336.011	1	0	1	.001	10	0	1	0	10	0	1
108			min	-74.029	3	0	1	-16.578	4	0	1	-.022	4	0	1
109		17	max	336.076	1	0	1	.001	10	0	1	0	10	0	1
110			min	-73.98	3	0	1	-16.634	4	0	1	-.023	4	0	1
111		18	max	336.141	1	0	1	.001	10	0	1	0	10	0	1
112			min	-73.932	3	0	1	-16.69	4	0	1	-.025	4	0	1
113		19	max	336.206	1	0	1	.001	10	0	1	0	10	0	1
114			min	-73.883	3	0	1	-16.746	4	0	1	-.026	4	0	1
115	M6	1	max	754.029	1	.631	6	.899	4	0	3	0	3	0	1
116			min	-1085.312	3	.144	15	-.242	3	0	5	0	2	0	1
117		2	max	754.135	1	.59	6	.803	4	0	3	0	4	0	15
118			min	-1085.232	3	.134	15	-.242	3	0	5	0	2	0	6
119		3	max	754.242	1	.549	6	.707	4	0	3	0	4	0	15
120			min	-1085.152	3	.125	15	-.242	3	0	5	0	2	0	6
121		4	max	754.348	1	.508	6	.61	4	0	3	0	4	0	15
122			min	-1085.072	3	.115	15	-.242	3	0	5	0	2	0	6
123		5	max	754.455	1	.466	6	.514	4	0	3	0	4	0	15
124			min	-1084.992	3	.105	15	-.242	3	0	5	0	2	0	6
125		6	max	754.562	1	.425	6	.417	4	0	3	0	4	0	15
126			min	-1084.912	3	.095	15	-.242	3	0	5	0	3	0	6
127		7	max	754.668	1	.387	2	.321	4	0	3	0	4	0	15
128			min	-1084.832	3	.086	15	-.242	3	0	5	0	3	0	6
129		8	max	754.775	1	.355	2	.224	4	0	3	0	4	0	15
130			min	-1084.753	3	.076	15	-.242	3	0	5	0	3	0	6
131		9	max	754.881	1	.323	2	.128	4	0	3	0	4	0	15
132			min	-1084.673	3	.066	15	-.242	3	0	5	0	3	0	6
133		10	max	754.988	1	.291	2	.04	14	0	3	0	4	0	15
134			min	-1084.593	3	.057	15	-.242	3	0	5	0	3	0	6
135		11	max	755.094	1	.258	2	.037	9	0	3	0	4	0	15
136			min	-1084.513	3	.047	15	-.242	3	0	5	0	3	0	6
137		12	max	755.201	1	.226	2	.037	9	0	3	0	4	0	15
138			min	-1084.433	3	.037	15	-.242	3	0	5	0	3	0	6
139		13	max	755.307	1	.194	2	.037	9	0	3	0	4	0	15
140			min	-1084.353	3	.024	12	-.272	5	0	5	0	3	0	2
141		14	max	755.414	1	.162	2	.037	9	0	3	0	4	0	15
142			min	-1084.273	3	.005	3	-.368	5	0	5	0	3	0	2



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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	755.52	1	.13	2	.037	9	0	3	0	4	0	15
144		min	-1084.193	3	-.019	3	-.465	5	0	5	0	3	0	2
145	16	max	755.627	1	.098	2	.037	9	0	3	0	4	0	15
146		min	-1084.113	3	-.043	3	-.561	5	0	5	0	3	0	2
147	17	max	755.733	1	.065	2	.037	9	0	3	0	4	0	15
148		min	-1084.033	3	-.067	3	-.658	5	0	5	0	3	0	2
149	18	max	755.84	1	.033	2	.037	9	0	3	0	4	0	15
150		min	-1083.953	3	-.091	3	-.754	5	0	5	0	3	0	2
151	19	max	755.947	1	.001	2	.037	9	0	3	0	14	0	15
152		min	-1083.874	3	-.115	3	-.85	5	0	5	0	3	0	2
153	M7	1	max	314.283	2	1.806	.013	3	0	9	0	4	0	2
154		min	-229.44	3	.429	15	-1.398	4	0	3	0	3	0	12
155	2	max	314.215	2	1.628	4	.013	3	0	9	0	4	0	2
156		min	-229.491	3	.387	15	-1.265	4	0	3	0	3	0	12
157	3	max	314.147	2	1.451	4	.013	3	0	9	0	1	0	2
158		min	-229.542	3	.345	15	-1.131	4	0	3	0	3	0	3
159	4	max	314.08	2	1.273	4	.013	3	0	9	0	1	0	2
160		min	-229.593	3	.303	15	-.998	4	0	3	0	3	0	3
161	5	max	314.012	2	1.095	4	.013	3	0	9	0	1	0	15
162		min	-229.644	3	.262	15	-.864	4	0	3	0	5	0	6
163	6	max	313.944	2	.918	4	.013	3	0	9	0	1	0	15
164		min	-229.694	3	.22	15	-.73	4	0	3	0	5	0	6
165	7	max	313.876	2	.74	4	.013	3	0	9	0	1	0	15
166		min	-229.745	3	.178	15	-.597	4	0	3	0	5	0	6
167	8	max	313.808	2	.562	4	.013	3	0	9	0	1	0	15
168		min	-229.796	3	.136	15	-.463	4	0	3	0	5	0	6
169	9	max	313.74	2	.385	4	.013	3	0	9	0	1	0	15
170		min	-229.847	3	.095	15	-.329	4	0	3	0	5	-.001	6
171	10	max	313.672	2	.21	2	.013	3	0	9	0	1	0	15
172		min	-229.898	3	.049	12	-.196	4	0	3	0	5	-.001	6
173	11	max	313.605	2	.071	2	.013	3	0	9	0	1	0	15
174		min	-229.949	3	-.035	3	-.062	4	0	3	0	5	-.001	6
175	12	max	313.537	2	-.031	15	.073	5	0	9	0	1	0	15
176		min	-230	3	-.149	6	-.016	1	0	3	0	5	-.001	6
177	13	max	313.469	2	-.072	15	.207	5	0	9	0	1	0	15
178		min	-230.051	3	-.326	6	-.016	1	0	3	0	5	-.001	6
179	14	max	313.401	2	-.114	15	.341	5	0	9	0	1	0	15
180		min	-230.102	3	-.504	6	-.016	1	0	3	0	5	-.001	6
181	15	max	313.333	2	-.156	15	.474	5	0	9	0	1	0	15
182		min	-230.153	3	-.682	6	-.016	1	0	3	0	5	0	6
183	16	max	313.265	2	-.198	15	.608	5	0	9	0	1	0	15
184		min	-230.203	3	-.859	6	-.016	1	0	3	0	5	0	6
185	17	max	313.197	2	-.239	15	.741	5	0	9	0	1	0	15
186		min	-230.254	3	-1.037	6	-.016	1	0	3	0	5	0	6
187	18	max	313.13	2	-.281	15	.875	5	0	9	0	1	0	15
188		min	-230.305	3	-1.215	6	-.016	1	0	3	0	5	0	6
189	19	max	313.062	2	-.323	15	1.009	5	0	9	0	9	0	1
190		min	-230.356	3	-1.392	6	-.016	1	0	3	0	3	0	1
191	M8	1	max	970.327	1	0	.146	9	0	1	0	4	0	1
192		min	-264.311	3	0	1	-16.074	4	0	1	0	3	0	1
193	2	max	970.391	1	0	1	.146	9	0	1	0	9	0	1
194		min	-264.263	3	0	1	-16.13	4	0	1	-.001	4	0	1
195	3	max	970.456	1	0	1	.146	9	0	1	0	9	0	1
196		min	-264.214	3	0	1	-16.187	4	0	1	-.003	4	0	1
197	4	max	970.521	1	0	1	.146	9	0	1	0	9	0	1
198		min	-264.166	3	0	1	-16.243	4	0	1	-.004	4	0	1
199	5	max	970.585	1	0	1	.146	9	0	1	0	9	0	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200		min	-264.117	3	0	1	-16.299	4	0	1	-.006	4	0	1
201	6	max	970.65	1	0	1	.146	9	0	1	0	9	0	1
202		min	-264.069	3	0	1	-16.355	4	0	1	-.007	4	0	1
203	7	max	970.715	1	0	1	.146	9	0	1	0	9	0	1
204		min	-264.02	3	0	1	-16.411	4	0	1	-.009	4	0	1
205	8	max	970.78	1	0	1	.146	9	0	1	0	9	0	1
206		min	-263.972	3	0	1	-16.467	4	0	1	-.01	4	0	1
207	9	max	970.844	1	0	1	.146	9	0	1	0	9	0	1
208		min	-263.923	3	0	1	-16.523	4	0	1	-.012	4	0	1
209	10	max	970.909	1	0	1	.146	9	0	1	0	9	0	1
210		min	-263.875	3	0	1	-16.579	4	0	1	-.013	4	0	1
211	11	max	970.974	1	0	1	.146	9	0	1	0	9	0	1
212		min	-263.826	3	0	1	-16.635	4	0	1	-.015	4	0	1
213	12	max	971.038	1	0	1	.146	9	0	1	0	9	0	1
214		min	-263.778	3	0	1	-16.691	4	0	1	-.016	4	0	1
215	13	max	971.103	1	0	1	.146	9	0	1	0	9	0	1
216		min	-263.729	3	0	1	-16.747	4	0	1	-.018	4	0	1
217	14	max	971.168	1	0	1	.146	9	0	1	0	9	0	1
218		min	-263.68	3	0	1	-16.803	4	0	1	-.019	4	0	1
219	15	max	971.233	1	0	1	.146	9	0	1	0	9	0	1
220		min	-263.632	3	0	1	-16.859	4	0	1	-.021	4	0	1
221	16	max	971.297	1	0	1	.146	9	0	1	0	9	0	1
222		min	-263.583	3	0	1	-16.916	4	0	1	-.022	4	0	1
223	17	max	971.362	1	0	1	.146	9	0	1	0	9	0	1
224		min	-263.535	3	0	1	-16.972	4	0	1	-.024	4	0	1
225	18	max	971.427	1	0	1	.146	9	0	1	0	9	0	1
226		min	-263.486	3	0	1	-17.028	4	0	1	-.025	4	0	1
227	19	max	971.491	1	0	1	.146	9	0	1	0	9	0	1
228		min	-263.438	3	0	1	-17.084	4	0	1	-.027	4	0	1
229	M10	1	max	244.314	1	.673	4	1.02	5	0	1	0	1	0
230		min	-314.679	3	.17	15	-.105	1	-.001	5	0	3	0	1
231	2	max	244.42	1	.632	4	.924	5	0	1	0	4	0	15
232		min	-314.599	3	.16	15	-.105	1	-.001	5	0	3	0	4
233	3	max	244.527	1	.591	4	.827	5	0	1	0	4	0	15
234		min	-314.519	3	.15	15	-.105	1	-.001	5	0	3	0	4
235	4	max	244.633	1	.549	4	.731	5	0	1	0	4	0	15
236		min	-314.439	3	.14	15	-.105	1	-.001	5	0	3	0	4
237	5	max	244.74	1	.508	4	.634	5	0	1	0	4	0	15
238		min	-314.359	3	.131	15	-.105	1	-.001	5	0	3	0	4
239	6	max	244.846	1	.467	4	.538	5	0	1	0	4	0	15
240		min	-314.279	3	.121	15	-.105	1	-.001	5	0	3	0	4
241	7	max	244.953	1	.426	4	.441	5	0	1	0	5	0	15
242		min	-314.199	3	.111	15	-.105	1	-.001	5	0	3	0	4
243	8	max	245.059	1	.384	4	.345	5	0	1	0	5	0	15
244		min	-314.119	3	.102	15	-.105	1	-.001	5	0	3	0	4
245	9	max	245.166	1	.343	4	.248	5	0	1	0	5	0	15
246		min	-314.039	3	.092	15	-.105	1	-.001	5	0	3	0	4
247	10	max	245.272	1	.302	4	.152	5	0	1	0	5	0	15
248		min	-313.96	3	.082	15	-.105	1	-.001	5	0	3	0	4
249	11	max	245.379	1	.261	4	.056	5	0	1	0	5	0	15
250		min	-313.88	3	.073	15	-.105	1	-.001	5	0	3	0	4
251	12	max	245.485	1	.219	4	-.002	10	0	1	0	5	0	15
252		min	-313.8	3	.063	15	-.105	1	-.001	5	0	3	0	4
253	13	max	245.592	1	.178	4	-.002	10	0	1	0	5	0	15
254		min	-313.72	3	.053	15	-.15	4	-.001	5	0	3	0	4
255	14	max	245.699	1	.137	4	-.002	10	0	1	0	5	0	15
256		min	-313.64	3	.043	15	-.246	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	245.805	1	.095	4	-.002	10	0	1	0	5	0	15
258		min	-313.56	3	.03	12	-.343	4	-.001	5	0	3	0	4
259	16	max	245.912	1	.054	4	-.002	10	0	1	0	5	0	15
260		min	-313.48	3	.01	9	-.439	4	-.001	5	0	3	0	4
261	17	max	246.018	1	.021	5	-.002	10	0	1	0	5	0	15
262		min	-313.4	3	-.017	9	-.535	4	-.001	5	0	3	0	4
263	18	max	246.125	1	.006	5	-.002	10	0	1	0	5	0	15
264		min	-313.32	3	-.044	9	-.632	4	-.001	5	0	3	0	4
265	19	max	246.231	1	-.005	15	-.002	10	0	1	0	5	0	15
266		min	-313.24	3	-.075	1	-.728	4	-.001	5	0	3	0	4
267	M11	1	max	95.338	2	1.792	.177	1	0	4	0	5	0	6
268		min	-86.327	3	.419	15	-1.267	5	0	10	0	1	0	15
269	2	max	95.27	2	1.614	6	.177	1	0	4	0	5	0	2
270		min	-86.378	3	.377	15	-1.133	5	0	10	0	1	0	15
271	3	max	95.202	2	1.437	6	.177	1	0	4	0	5	0	2
272		min	-86.429	3	.335	15	-1	5	0	10	0	1	0	3
273	4	max	95.135	2	1.259	6	.177	1	0	4	0	3	0	15
274		min	-86.48	3	.293	15	-.866	5	0	10	0	1	0	4
275	5	max	95.067	2	1.082	6	.177	1	0	4	0	3	0	15
276		min	-86.53	3	.251	15	-.733	5	0	10	0	1	0	4
277	6	max	94.999	2	.904	6	.177	1	0	4	0	3	0	15
278		min	-86.581	3	.21	15	-.599	5	0	10	0	1	0	4
279	7	max	94.931	2	.726	6	.177	1	0	4	0	3	0	15
280		min	-86.632	3	.168	15	-.465	5	0	10	0	1	0	4
281	8	max	94.863	2	.549	6	.177	1	0	4	0	3	0	15
282		min	-86.683	3	.126	15	-.332	5	0	10	0	4	-.001	4
283	9	max	94.795	2	.371	6	.177	1	0	4	0	3	0	15
284		min	-86.734	3	.084	15	-.198	5	0	10	0	4	-.001	4
285	10	max	94.727	2	.193	6	.177	1	0	4	0	3	0	15
286		min	-86.785	3	.043	15	-.064	5	0	10	0	4	-.001	4
287	11	max	94.66	2	.036	2	.177	1	0	4	0	3	0	15
288		min	-86.836	3	-.016	3	-.027	3	0	10	0	4	-.001	4
289	12	max	94.592	2	-.041	15	.241	4	0	4	0	3	0	15
290		min	-86.887	3	-.162	4	-.027	3	0	10	0	4	-.001	4
291	13	max	94.524	2	-.083	15	.375	4	0	4	0	3	0	15
292		min	-86.938	3	-.34	4	-.027	3	0	10	0	4	-.001	4
293	14	max	94.456	2	-.124	15	.508	4	0	4	0	3	0	15
294		min	-86.989	3	-.518	4	-.027	3	0	10	0	5	-.001	4
295	15	max	94.388	2	-.166	15	.642	4	0	4	0	3	0	15
296		min	-87.039	3	-.695	4	-.027	3	0	10	0	5	0	4
297	16	max	94.32	2	-.208	15	.776	4	0	4	0	3	0	15
298		min	-87.09	3	-.873	4	-.027	3	0	10	0	10	0	4
299	17	max	94.252	2	-.25	15	.909	4	0	4	0	3	0	15
300		min	-87.141	3	-1.051	4	-.027	3	0	10	0	10	0	4
301	18	max	94.185	2	-.291	15	1.043	4	0	4	0	4	0	15
302		min	-87.192	3	-1.228	4	-.027	3	0	10	0	10	0	4
303	19	max	94.117	2	-.333	15	1.176	4	0	4	0	4	0	1
304		min	-87.243	3	-1.406	4	-.027	3	0	10	0	10	0	1
305	M12	1	max	335.186	1	0	.767	1	0	1	0	4	0	1
306		min	-74.324	3	0	1	-14.78	5	0	1	0	3	0	1
307	2	max	335.251	1	0	1	.767	1	0	1	0	1	0	1
308		min	-74.275	3	0	1	-14.836	5	0	1	-.001	5	0	1
309	3	max	335.316	1	0	1	.767	1	0	1	0	1	0	1
310		min	-74.227	3	0	1	-14.892	5	0	1	-.003	5	0	1
311	4	max	335.38	1	0	1	.767	1	0	1	0	1	0	1
312		min	-74.178	3	0	1	-14.948	5	0	1	-.004	5	0	1
313	5	max	335.445	1	0	1	.767	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-74.13	3	0	1	-15.004	5	0	1	-.005	5	0	1
315	6	max	335.51	1	0	1	.767	1	0	1	0	1	0	1
316		min	-74.081	3	0	1	-15.06	5	0	1	-.007	5	0	1
317	7	max	335.575	1	0	1	.767	1	0	1	0	1	0	1
318		min	-74.033	3	0	1	-15.116	5	0	1	-.008	5	0	1
319	8	max	335.639	1	0	1	.767	1	0	1	0	1	0	1
320		min	-73.984	3	0	1	-15.173	5	0	1	-.009	5	0	1
321	9	max	335.704	1	0	1	.767	1	0	1	0	1	0	1
322		min	-73.935	3	0	1	-15.229	5	0	1	-.011	5	0	1
323	10	max	335.769	1	0	1	.767	1	0	1	0	1	0	1
324		min	-73.887	3	0	1	-15.285	5	0	1	-.012	5	0	1
325	11	max	335.833	1	0	1	.767	1	0	1	0	1	0	1
326		min	-73.838	3	0	1	-15.341	5	0	1	-.013	5	0	1
327	12	max	335.898	1	0	1	.767	1	0	1	0	1	0	1
328		min	-73.79	3	0	1	-15.397	5	0	1	-.015	5	0	1
329	13	max	335.963	1	0	1	.767	1	0	1	0	1	0	1
330		min	-73.741	3	0	1	-15.453	5	0	1	-.016	5	0	1
331	14	max	336.028	1	0	1	.767	1	0	1	0	1	0	1
332		min	-73.693	3	0	1	-15.509	5	0	1	-.018	5	0	1
333	15	max	336.092	1	0	1	.767	1	0	1	0	1	0	1
334		min	-73.644	3	0	1	-15.565	5	0	1	-.019	5	0	1
335	16	max	336.157	1	0	1	.767	1	0	1	.001	1	0	1
336		min	-73.596	3	0	1	-15.621	5	0	1	-.02	5	0	1
337	17	max	336.222	1	0	1	.767	1	0	1	.001	1	0	1
338		min	-73.547	3	0	1	-15.677	5	0	1	-.022	5	0	1
339	18	max	336.286	1	0	1	.767	1	0	1	.001	1	0	1
340		min	-73.499	3	0	1	-15.733	5	0	1	-.023	5	0	1
341	19	max	336.351	1	0	1	.767	1	0	1	.001	1	0	1
342		min	-73.45	3	0	1	-15.789	5	0	1	-.025	5	0	1
343	M1	1	max	67.486	1	336.373	3	-.09	10	0	.033	1	.015	2
344		min	4.707	10	-246.807	1	-17.217	4	0	3	0	10	-.016	3
345	2	max	67.581	1	336.176	3	-.09	10	0	1	.03	1	.068	1
346		min	4.787	10	-247.069	1	-16.981	1	0	3	0	10	-.089	3
347	3	max	54.572	1	4.677	4	-.089	10	0	5	.026	1	.121	1
348		min	-.723	10	-20.741	3	-16.888	1	0	1	0	10	-.161	3
349	4	max	54.668	1	4.416	14	-.089	10	0	5	.022	1	.122	1
350		min	-.644	10	-20.938	3	-16.888	1	0	1	0	10	-.156	3
351	5	max	54.763	1	4.158	14	-.089	10	0	5	.018	1	.124	2
352		min	-.564	10	-21.135	3	-16.888	1	0	1	0	10	-.152	3
353	6	max	54.859	1	3.9	14	-.089	10	0	5	.015	1	.128	2
354		min	-.484	10	-21.331	3	-16.888	1	0	1	0	10	-.147	3
355	7	max	54.954	1	3.642	14	-.089	10	0	5	.011	1	.132	2
356		min	-.405	10	-21.528	3	-16.888	1	0	1	0	10	-.142	3
357	8	max	55.05	1	3.384	14	-.089	10	0	5	.007	1	.136	2
358		min	-.325	10	-21.725	3	-16.888	1	0	1	0	10	-.138	3
359	9	max	55.145	1	3.127	14	-.089	10	0	5	.004	1	.14	2
360		min	-.246	10	-21.922	3	-16.888	1	0	1	0	10	-.133	3
361	10	max	55.241	1	2.869	14	-.089	10	0	5	.001	3	.144	2
362		min	-.166	10	-22.118	3	-16.888	1	0	1	0	10	-.128	3
363	11	max	55.336	1	2.611	14	-.089	10	0	5	0	3	.149	2
364		min	-.086	10	-22.315	3	-16.888	1	0	1	-.004	1	-.123	3
365	12	max	55.432	1	2.368	9	-.089	10	0	5	0	10	.153	2
366		min	-.007	10	-22.512	3	-16.888	1	0	1	-.007	1	-.119	3
367	13	max	55.527	1	2.149	9	-.089	10	0	5	0	10	.158	2
368		min	.073	10	-22.709	3	-16.888	1	0	1	-.011	1	-.114	3
369	14	max	55.623	1	1.93	9	-.089	10	0	5	0	10	.162	2
370		min	.152	10	-22.906	3	-16.888	1	0	1	-.015	1	-.109	3



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	55.718	1	1.712	9	-.089	10	0	5	0	10	.167	2
372		min	.232	10	-23.102	3	-16.888	1	0	1	-.018	1	-.104	3
373	16	max	80.073	2	46.192	2	-.089	10	0	1	0	10	.171	2
374		min	-30.389	3	-86.202	3	-17.039	1	0	5	-.022	1	-.098	3
375	17	max	80.169	2	45.93	2	-.089	10	0	1	0	10	.161	2
376		min	-30.317	3	-86.399	3	-17.039	1	0	5	-.026	1	-.079	3
377	18	max	-3.053	12	331.586	2	-.088	10	0	5	0	10	.09	2
378		min	-67.549	1	-156.921	3	-26.659	4	0	2	-.03	1	-.046	3
379	19	max	-3.005	12	331.324	2	-.088	10	0	5	0	10	.018	2
380		min	-67.453	1	-157.117	3	-26.417	4	0	2	-.033	1	-.012	3
381	M5	1	max	165.794	1	1062.355	3	0	10	0	.032	4	.033	3
382		min	-.123	3	-774.034	1	-58.185	3	0	5	0	10	-.029	2
383	2	max	165.89	1	1062.158	3	0	10	0	9	.028	4	.139	1
384		min	-.052	3	-774.296	1	-58.185	3	0	5	-.003	3	-.198	3
385	3	max	124.501	1	6.019	9	6.113	3	0	3	.023	4	.304	1
386		min	-.114	15	-64.645	3	-17.961	4	0	4	-.015	3	-.423	3
387	4	max	124.597	1	5.801	9	6.113	3	0	3	.019	4	.309	1
388		min	-.085	15	-64.841	3	-17.719	4	0	4	-.014	3	-.409	3
389	5	max	124.692	1	5.582	9	6.113	3	0	3	.016	4	.314	1
390		min	-.057	15	-65.038	3	-17.477	4	0	4	-.013	3	-.395	3
391	6	max	124.788	1	5.363	9	6.113	3	0	3	.012	4	.323	2
392		min	-.028	15	-65.235	3	-17.235	4	0	4	-.012	3	-.381	3
393	7	max	124.883	1	5.145	9	6.113	3	0	3	.008	4	.335	2
394		min	.001	15	-65.432	3	-16.993	4	0	4	-.01	3	-.367	3
395	8	max	124.979	1	4.926	9	6.113	3	0	3	.004	4	.348	2
396		min	.03	15	-65.629	3	-16.751	4	0	4	-.009	3	-.352	3
397	9	max	125.074	1	4.707	9	6.113	3	0	3	0	4	.36	2
398		min	.059	15	-65.825	3	-16.509	4	0	4	-.008	3	-.338	3
399	10	max	125.17	1	4.489	9	6.113	3	0	3	0	11	.373	2
400		min	.088	15	-66.022	3	-16.267	4	0	4	-.006	3	-.324	3
401	11	max	125.265	1	4.27	9	6.113	3	0	3	0	2	.385	2
402		min	.116	15	-66.219	3	-16.025	4	0	4	-.006	4	-.309	3
403	12	max	125.361	1	4.051	9	6.113	3	0	3	0	2	.398	2
404		min	.145	15	-66.416	3	-15.783	4	0	4	-.01	4	-.295	3
405	13	max	125.456	1	3.833	9	6.113	3	0	3	0	2	.411	2
406		min	.174	15	-66.613	3	-15.541	4	0	4	-.013	4	-.281	3
407	14	max	125.552	1	3.614	9	6.113	3	0	3	0	2	.424	2
408		min	.203	15	-66.809	3	-15.299	4	0	4	-.016	4	-.266	3
409	15	max	125.647	1	3.395	9	6.113	3	0	3	0	3	.437	2
410		min	.232	15	-67.006	3	-15.057	4	0	4	-.02	4	-.252	3
411	16	max	250.608	2	167.627	2	6.091	3	0	3	.001	3	.447	2
412		min	-94.932	3	-233.166	3	-13.813	4	0	4	-.023	4	-.236	3
413	17	max	250.704	2	167.365	2	6.091	3	0	3	.003	3	.411	2
414		min	-94.861	3	-233.363	3	-13.571	4	0	4	-.026	4	-.185	3
415	18	max	-3.298	12	1039.22	2	5.645	3	0	4	.004	3	.189	2
416		min	-165.964	1	-482.613	3	-29.062	5	0	9	-.032	4	-.081	3
417	19	max	-3.25	12	1038.958	2	5.645	3	0	4	.005	3	.023	3
418		min	-165.868	1	-482.81	3	-28.82	5	0	9	-.039	4	-.036	2
419	M9	1	max	67.383	1	336.326	3	121.896	4	0	3	0	.015	2
420		min	.355	15	-246.806	1	.09	10	0	1	-.033	1	-.016	3
421	2	max	67.479	1	336.129	3	122.138	4	0	3	.025	5	.068	1
422		min	.383	15	-247.068	1	.09	10	0	1	-.029	1	-.089	3
423	3	max	54.827	1	4.319	9	16.648	1	0	1	.049	5	.12	1
424		min	-.394	10	-20.667	3	-22.999	5	0	10	-.025	1	-.161	3
425	4	max	54.922	1	4.1	9	16.648	1	0	1	.044	5	.121	1
426		min	-.315	10	-20.864	3	-22.757	5	0	10	-.022	1	-.156	3
427	5	max	55.018	1	3.882	9	16.648	1	0	1	.039	5	.123	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-.235	10	-21.061	3	-22.515	5	0	10	-.018	1	-.152	3
429		6	max	55.113	1	3.663	9	16.648	1	0	1	.034	5	.128	2
430			min	-.155	10	-21.258	3	-22.273	5	0	10	-.015	1	-.147	3
431		7	max	55.209	1	3.444	9	16.648	1	0	1	.03	5	.132	2
432			min	-.076	10	-21.455	3	-22.031	5	0	10	-.011	1	-.142	3
433		8	max	55.304	1	3.226	9	16.648	1	0	1	.025	5	.136	2
434			min	.004	10	-21.651	3	-21.789	5	0	10	-.007	1	-.138	3
435		9	max	55.4	1	3.007	9	16.648	1	0	1	.02	5	.14	2
436			min	.083	10	-21.848	3	-21.547	5	0	10	-.004	1	-.133	3
437		10	max	55.495	1	2.788	9	16.648	1	0	1	.015	4	.144	2
438			min	.163	10	-22.045	3	-21.305	5	0	10	0	1	-.128	3
439		11	max	55.591	1	2.57	9	16.648	1	0	1	.012	4	.149	2
440			min	.242	10	-22.242	3	-21.063	5	0	10	0	10	-.123	3
441		12	max	55.686	1	2.351	9	16.648	1	0	1	.008	4	.153	2
442			min	.322	10	-22.439	3	-20.821	5	0	10	0	10	-.119	3
443		13	max	55.782	1	2.132	9	16.648	1	0	1	.011	1	.158	2
444			min	.402	10	-22.635	3	-20.579	5	0	10	0	10	-.114	3
445		14	max	55.877	1	1.914	9	16.648	1	0	1	.014	1	.162	2
446			min	.481	10	-22.832	3	-20.337	5	0	10	-.003	5	-.109	3
447		15	max	55.973	1	1.695	9	16.648	1	0	1	.018	1	.167	2
448			min	.561	10	-23.029	3	-20.095	5	0	10	-.007	5	-.104	3
449		16	max	80.176	2	45.911	2	16.811	1	0	10	.022	1	.171	2
450			min	-31.106	3	-86.603	3	-18.716	5	0	4	-.01	5	-.098	3
451		17	max	80.271	2	45.648	2	16.811	1	0	10	.026	1	.161	2
452			min	-31.035	3	-86.8	3	-18.474	5	0	4	-.014	5	-.079	3
453		18	max	8.63	5	331.586	2	17.571	1	0	2	.029	1	.09	2
454			min	-67.437	1	-156.915	3	-33.003	5	0	3	-.022	5	-.046	3
455		19	max	8.674	5	331.324	2	17.571	1	0	2	.033	1	.018	2
456			min	-67.342	1	-157.112	3	-32.761	5	0	3	-.029	5	-.012	3
457	M13	1	max	121.895	4	246.584	1	-.355	15	.015	2	.033	1	0	1
458			min	.09	10	-336.345	3	-67.381	1	-.016	3	0	10	0	3
459		2	max	117.219	4	176.267	1	.202	15	.015	2	.011	3	.136	3
460			min	.09	10	-239.956	3	-50.677	1	-.016	3	-.002	10	-.1	1
461		3	max	112.543	4	105.95	1	1.06	5	.015	2	.008	3	.227	3
462			min	.09	10	-143.567	3	-33.974	1	-.016	3	-.015	1	-.166	1
463		4	max	107.868	4	35.634	1	1.921	5	.015	2	.006	3	.272	3
464			min	.09	10	-47.179	3	-17.27	1	-.016	3	-.027	1	-.2	1
465		5	max	103.192	4	49.21	3	2.782	5	.015	2	.004	3	.271	3
466			min	.09	10	-34.683	1	-3.826	3	-.016	3	-.031	1	-.2	1
467		6	max	98.516	4	145.599	3	16.137	1	.015	2	.004	5	.225	3
468			min	.09	10	-105	1	-3.012	3	-.016	3	-.027	1	-.167	1
469		7	max	93.84	4	241.987	3	32.841	1	.015	2	.006	5	.134	3
470			min	.09	10	-175.316	1	-2.197	3	-.016	3	-.016	1	-.101	1
471		8	max	89.164	4	338.376	3	49.544	1	.015	2	.009	4	0	4
472			min	.09	10	-245.633	1	-1.382	3	-.016	3	0	12	-.003	3
473		9	max	84.489	4	434.765	3	66.248	1	.015	2	.031	1	.131	1
474			min	.09	10	-315.95	1	-.568	3	-.016	3	0	3	-.186	3
475		10	max	79.813	4	317.871	12	82.951	1	.015	2	.066	1	.297	1
476			min	.09	10	-531.153	3	.383	3	-.016	3	-.018	5	-.414	3
477		11	max	54.604	4	315.95	1	6.778	5	.016	3	.031	1	.131	1
478			min	.09	10	-434.764	3	-66.145	1	-.015	2	-.015	5	-.186	3
479		12	max	49.929	4	245.633	1	7.639	5	.016	3	.004	2	0	4
480			min	.09	10	-338.376	3	-49.442	1	-.015	2	-.011	5	-.003	3
481		13	max	45.253	4	175.316	1	8.5	5	.016	3	0	10	.134	3
482			min	.09	10	-241.987	3	-32.738	1	-.015	2	-.016	1	-.101	1
483		14	max	40.577	4	105	1	9.362	5	.016	3	-.002	10	.225	3
484			min	.09	10	-145.599	3	-16.035	1	-.015	2	-.027	1	-.167	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	35.901	4	34.683	1	10.711	4	.016	3	.001	5	.271	3
486			min	.09	10	-49.21	3	-1.95	2	-.015	2	-.031	1	-.2	1
487		16	max	31.226	4	47.179	3	17.372	1	.016	3	.006	5	.272	3
488			min	.09	10	-35.634	1	.094	10	-.015	2	-.027	1	-.2	1
489		17	max	26.55	4	143.567	3	34.076	1	.016	3	.012	5	.227	3
490			min	.09	10	-105.95	1	1.632	10	-.015	2	-.015	1	-.166	1
491		18	max	21.874	4	239.956	3	50.779	1	.016	3	.019	4	.136	3
492			min	.09	10	-176.267	1	3.17	10	-.015	2	-.002	10	-.1	1
493		19	max	17.198	4	336.345	3	67.483	1	.016	3	.033	1	0	1
494			min	.09	10	-246.584	1	4.707	10	-.015	2	0	10	0	3
495	M16	1	max	32.75	5	331.425	2	8.674	5	.012	3	.033	1	0	2
496			min	-17.544	1	-157.13	3	-67.345	1	-.018	2	-.029	5	0	3
497		2	max	28.074	5	236.837	2	9.535	5	.012	3	.005	1	.064	3
498			min	-17.544	1	-112.846	3	-50.641	1	-.018	2	-.024	5	-.134	2
499		3	max	23.398	5	142.248	2	10.396	5	.012	3	0	12	.107	3
500			min	-17.544	1	-68.563	3	-33.938	1	-.018	2	-.022	4	-.224	2
501		4	max	18.722	5	47.66	2	11.257	5	.012	3	-.001	12	.128	3
502			min	-17.544	1	-24.279	3	-17.234	1	-.018	2	-.027	1	-.269	2
503		5	max	14.047	5	20.005	3	12.118	5	.012	3	-.002	12	.13	3
504			min	-17.544	1	-46.929	2	-2.238	3	-.018	2	-.031	1	-.269	2
505		6	max	9.371	5	64.288	3	16.173	1	.012	3	-.002	10	.11	3
506			min	-17.544	1	-141.517	2	-1.423	3	-.018	2	-.027	1	-.224	2
507		7	max	4.695	5	108.572	3	32.876	1	.012	3	.003	5	.069	3
508			min	-17.544	1	-236.105	2	-.608	3	-.018	2	-.016	1	-.135	2
509		8	max	2.149	3	152.855	3	49.58	1	.012	3	.01	4	.007	3
510			min	-17.544	1	-330.694	2	.206	3	-.018	2	-.005	3	-.001	2
511		9	max	2.149	3	197.139	3	66.283	1	.012	3	.031	1	.177	2
512			min	-17.544	1	-425.282	2	.823	12	-.018	2	-.005	3	-.076	3
513		10	max	19.494	5	-8.383	15	82.987	1	.005	14	.066	1	.4	2
514			min	-17.544	1	-519.871	2	-2.702	3	-.018	2	-.004	3	-.179	3
515		11	max	14.819	5	425.282	2	5.718	5	.018	2	.031	1	.177	2
516			min	-17.506	1	-197.139	3	-66.172	1	-.012	3	-.011	5	-.076	3
517		12	max	10.143	5	330.694	2	6.579	5	.018	2	.004	2	.007	3
518			min	-17.506	1	-152.855	3	-49.468	1	-.012	3	-.008	5	-.001	2
519		13	max	5.467	5	236.105	2	7.44	5	.018	2	0	10	.069	3
520			min	-17.506	1	-108.572	3	-32.765	1	-.012	3	-.016	1	-.135	2
521		14	max	.791	5	141.517	2	8.302	5	.018	2	0	12	.11	3
522			min	-17.506	1	-64.288	3	-16.061	1	-.012	3	-.027	1	-.224	2
523		15	max	-.088	10	46.929	2	9.625	4	.018	2	.003	5	.13	3
524			min	-17.506	1	-20.005	3	-1.962	2	-.012	3	-.031	1	-.269	2
525		16	max	-.088	10	24.279	3	17.346	1	.018	2	.007	5	.128	3
526			min	-17.506	1	-47.66	2	.088	10	-.012	3	-.027	1	-.269	2
527		17	max	-.088	10	68.563	3	34.049	1	.018	2	.012	5	.107	3
528			min	-17.506	1	-142.248	2	1.626	10	-.012	3	-.015	1	-.224	2
529		18	max	-.088	10	112.846	3	50.753	1	.018	2	.019	4	.064	3
530			min	-21.762	4	-236.837	2	2.461	12	-.012	3	-.002	10	-.134	2
531		19	max	-.088	10	157.13	3	67.456	1	.018	2	.033	1	0	2
532			min	-26.438	4	-331.425	2	3.004	12	-.012	3	0	10	0	5
533	M15	1	max	0	1	.876	3	.109	3	0	1	0	1	0	1
534			min	-76.648	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.778	3	.109	3	0	1	0	1	0	1
536			min	-76.708	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.681	3	.109	3	0	1	0	1	0	1
538			min	-76.768	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.584	3	.109	3	0	1	0	1	0	1
540			min	-76.827	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.487	3	.109	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-76.887	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.389	3	.109	3	0	1	0	1	0	1
544			min	-76.947	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.292	3	.109	3	0	1	0	3	0	1
546			min	-77.006	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.195	3	.109	3	0	1	0	3	0	1
548			min	-77.066	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.097	3	.109	3	0	1	0	3	0	1
550			min	-77.126	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.109	3	0	1	0	3	0	1
552			min	-77.185	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.109	3	0	1	0	3	0	1
554			min	-77.245	3	-.097	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.109	3	0	1	0	3	0	1
556			min	-77.305	3	-.195	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.109	3	0	1	0	3	0	1
558			min	-77.364	3	-.292	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.109	3	0	1	0	3	0	1
560			min	-77.424	3	-.389	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.109	3	0	1	0	3	0	1
562			min	-77.484	3	-.487	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.109	3	0	1	0	3	0	1
564			min	-77.543	3	-.584	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.109	3	0	1	0	3	0	1
566			min	-77.603	3	-.681	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.109	3	0	1	0	3	0	1
568			min	-77.663	3	-.778	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.109	3	0	1	0	3	0	1
570			min	-77.722	3	-.876	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.108	4	.255	4	0	3	0	3	0	1
572			min	-171.905	4	0	2	-.051	3	0	1	0	4	0	1
573		2	max	0	2	1.874	4	.231	4	0	3	0	3	0	2
574			min	-171.914	4	0	2	-.051	3	0	1	0	4	0	4
575		3	max	0	2	1.64	4	.206	4	0	3	0	3	0	2
576			min	-171.922	4	0	2	-.051	3	0	1	0	4	-.001	4
577		4	max	0	2	1.406	4	.182	4	0	3	0	3	0	2
578			min	-171.931	4	0	2	-.051	3	0	1	0	1	-.001	4
579		5	max	0	2	1.171	4	.158	4	0	3	0	3	0	2
580			min	-171.939	4	0	2	-.051	3	0	1	0	1	-.002	4
581		6	max	0	2	.937	4	.133	4	0	3	0	3	0	2
582			min	-171.947	4	0	2	-.051	3	0	1	0	1	-.002	4
583		7	max	0	2	.703	4	.109	4	0	3	0	3	0	2
584			min	-171.956	4	0	2	-.051	3	0	1	0	1	-.002	4
585		8	max	0	2	.469	4	.084	4	0	3	0	5	0	2
586			min	-171.964	4	0	2	-.051	3	0	1	0	1	-.003	4
587		9	max	0	2	.234	4	.06	4	0	3	0	5	0	2
588			min	-171.973	4	0	2	-.051	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.044	1	0	3	0	5	0	2
590			min	-171.981	4	0	1	-.051	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.044	1	0	3	0	5	0	2
592			min	-171.989	4	-.234	4	-.051	3	0	1	0	1	-.003	4
593		12	max	.043	11	0	2	.044	1	0	3	0	5	0	2
594			min	-171.998	4	-.469	4	-.051	3	0	1	0	1	-.003	4
595		13	max	.11	11	0	2	.044	1	0	3	0	5	0	2
596			min	-172.006	4	-.703	4	-.051	3	0	1	0	9	-.002	4
597		14	max	.176	11	0	2	.044	1	0	3	0	5	0	2
598			min	-172.015	4	-.937	4	-.067	5	0	1	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.242	11	0	2	.044	1	0	3	0	4	0	2
600		min	-172.023	4	-1.171	4	-.091	5	0	1	0	3	-.002	4
601	16	max	.309	11	0	2	.044	1	0	3	0	4	0	2
602		min	-172.031	4	-1.406	4	-.115	5	0	1	0	3	-.001	4
603	17	max	.375	11	0	2	.044	1	0	3	0	1	0	2
604		min	-172.04	4	-1.64	4	-.14	5	0	1	0	3	-.001	4
605	18	max	.441	11	0	2	.044	1	0	3	0	1	0	2
606		min	-172.048	4	-1.874	4	-.164	5	0	1	0	3	0	4
607	19	max	.507	11	0	2	.044	1	0	3	0	1	0	1
608		min	-172.085	5	-2.108	4	-.189	5	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.007	2	.003	1	8.767e-4	5	NC	3	NC	1	
2			min	-.003	3	-.007	3	-.009	5	-2.542e-4	1	4456.915	2	NC	1	
3			2	max	.002	1	.007	2	.003	1	8.976e-4	5	NC	3	NC	1
4				min	-.003	3	-.006	3	-.009	5	-2.433e-4	1	4835.076	2	NC	1
5			3	max	.002	1	.006	2	.002	1	9.185e-4	5	NC	3	NC	1
6				min	-.003	3	-.006	3	-.008	5	-2.324e-4	1	5279.862	2	NC	1
7			4	max	.002	1	.006	2	.002	1	9.394e-4	5	NC	3	NC	1
8				min	-.002	3	-.006	3	-.008	5	-2.215e-4	1	5806.549	2	NC	1
9			5	max	.002	1	.005	2	.002	1	9.603e-4	5	NC	1	NC	1
10				min	-.002	3	-.006	3	-.008	5	-2.106e-4	1	6435.272	2	NC	1
11			6	max	.001	1	.005	2	.002	1	9.812e-4	5	NC	1	NC	1
12				min	-.002	3	-.005	3	-.007	5	-1.997e-4	1	7192.999	2	NC	1
13			7	max	.001	1	.004	2	.002	1	1.002e-3	5	NC	1	NC	1
14				min	-.002	3	-.005	3	-.007	5	-1.888e-4	1	8116.532	2	NC	1
15			8	max	.001	1	.004	2	.001	1	1.023e-3	5	NC	1	NC	1
16				min	-.002	3	-.005	3	-.007	5	-1.779e-4	1	9257.202	2	NC	1
17			9	max	.001	1	.003	2	.001	1	1.044e-3	5	NC	1	NC	1
18				min	-.002	3	-.004	3	-.006	5	-1.67e-4	1	NC	1	NC	1
19			10	max	0	1	.003	2	0	1	1.065e-3	5	NC	1	NC	1
20				min	-.001	3	-.004	3	-.006	5	-1.561e-4	1	NC	1	NC	1
21		11	max	0	1	.002	2	0	1	1.086e-3	5	NC	1	NC	1	
22			min	-.001	3	-.004	3	-.005	5	-1.452e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	0	1	1.107e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.005	5	-1.343e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	1.127e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.004	5	-1.234e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.148e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.003	5	-1.125e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.169e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.003	5	-1.016e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.19e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-9.067e-5	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.211e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.001	5	-7.977e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.232e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-6.887e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.253e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-5.796e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.669e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.758e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.003	5	3.56e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-5.782e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	4.451e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-5.806e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.009	5	5.341e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-5.829e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.012	4	6.232e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-5.853e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.015	4	7.123e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.877e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.018	4	8.014e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-5.9e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.021	4	8.905e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-5.924e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.024	4	9.796e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-5.948e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.027	4	1.069e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.971e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.029	4	1.158e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-5.995e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.032	4	1.247e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-6.019e-4	5	NC	1	NC	1
63		13	max	0	3	.004	2	.034	4	1.336e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-6.042e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.037	4	1.425e-4	1	NC	1	NC	1
66			min	0	2	-.008	3	0	10	-6.066e-4	5	NC	1	NC	1
67		15	max	0	3	.005	2	.039	4	1.514e-4	1	NC	1	NC	1
68			min	0	2	-.008	3	0	10	-6.09e-4	5	8702.783	2	NC	1
69		16	max	0	3	.006	2	.042	4	1.603e-4	1	NC	1	NC	1
70			min	0	2	-.008	3	0	10	-6.113e-4	5	7418.916	2	NC	1
71		17	max	0	3	.007	2	.044	4	1.692e-4	1	NC	3	NC	1
72			min	0	2	-.008	3	0	10	-6.137e-4	5	6414.852	2	NC	1
73		18	max	0	3	.008	2	.046	4	1.781e-4	1	NC	3	NC	1
74			min	-.001	2	-.008	3	0	10	-6.161e-4	5	5622.472	2	NC	1
75		19	max	0	3	.009	2	.048	4	1.871e-4	1	NC	3	NC	1
76			min	-.001	2	-.008	3	0	10	-6.184e-4	5	4992.688	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	2.406e-3	5	NC	1	NC	1
78			min	0	3	-.007	3	-.051	4	-2.112e-4	1	NC	1	380.018	4
79		2	max	.002	1	.008	2	0	10	2.406e-3	5	NC	1	NC	1
80			min	0	3	-.007	3	-.047	4	-2.112e-4	1	NC	1	414.226	4
81		3	max	.001	1	.007	2	0	10	2.406e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.042	4	-2.112e-4	1	NC	1	454.935	4
83		4	max	.001	1	.007	2	0	10	2.406e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.038	4	-2.112e-4	1	NC	1	503.856	4
85		5	max	.001	1	.007	2	0	10	2.406e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.034	4	-2.112e-4	1	NC	1	563.323	4
87		6	max	.001	1	.006	2	0	10	2.406e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.03	4	-2.112e-4	1	NC	1	636.577	4
89		7	max	.001	1	.006	2	0	10	2.406e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.027	4	-2.112e-4	1	NC	1	728.239	4
91		8	max	0	1	.005	2	0	10	2.406e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.023	4	-2.112e-4	1	NC	1	845.069	4
93		9	max	0	1	.005	2	0	10	2.406e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.019	4	-2.112e-4	1	NC	1	997.295	4
95		10	max	0	1	.004	2	0	10	2.406e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.016	4	-2.112e-4	1	NC	1	1200.994	4
97		11	max	0	1	.004	2	0	10	2.406e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.013	4	-2.112e-4	1	NC	1	1482.642	4
99		12	max	0	1	.003	2	0	10	2.406e-3	5	NC	1	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.003	3	-.01	4	-2.112e-4	1	NC	1	1888.418	4
101		max	0	1	.003	2	0	10	2.406e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.008	4	-2.112e-4	1	NC	1	2504.938	4
103		max	0	1	.002	2	0	10	2.406e-3	5	NC	1	NC	1
104		min	0	3	-.002	3	-.006	4	-2.112e-4	1	NC	1	3510.854	4
105		max	0	1	.002	2	0	10	2.406e-3	5	NC	1	NC	1
106		min	0	3	-.002	3	-.004	4	-2.112e-4	1	NC	1	5325.855	4
107		max	0	1	.001	2	0	10	2.406e-3	5	NC	1	NC	1
108		min	0	3	-.001	3	-.002	4	-2.112e-4	1	NC	1	9142.954	4
109		max	0	1	0	2	0	10	2.406e-3	5	NC	1	NC	1
110		min	0	3	0	3	0	4	-2.112e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.406e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-2.112e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.406e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-2.112e-4	1	NC	1	NC	1
115	M6	max	.006	1	.02	2	0	9	9.346e-4	4	NC	3	NC	1
116		min	-.009	3	-.016	3	-.009	5	-8.429e-8	2	1688.984	2	7712.333	3
117		max	.006	1	.018	2	0	9	9.552e-4	4	NC	3	NC	1
118		min	-.008	3	-.016	3	-.009	5	-4.849e-7	11	1806.859	2	8244.223	3
119		max	.005	1	.017	2	0	9	9.759e-4	4	NC	3	NC	1
120		min	-.008	3	-.015	3	-.008	5	-1.396e-6	11	1941.903	2	8869.015	3
121		max	.005	1	.016	2	0	9	9.966e-4	4	NC	3	NC	1
122		min	-.007	3	-.014	3	-.008	5	-2.429e-6	1	2097.586	2	9607.361	3
123		max	.005	1	.015	2	0	9	1.017e-3	4	NC	3	NC	1
124		min	-.007	3	-.013	3	-.008	5	-4.447e-6	1	2278.38	2	NC	1
125		max	.004	1	.013	2	0	9	1.038e-3	4	NC	3	NC	1
126		min	-.006	3	-.012	3	-.007	5	-6.465e-6	1	2490.149	2	NC	1
127		max	.004	1	.012	2	0	9	1.059e-3	4	NC	3	NC	1
128		min	-.006	3	-.011	3	-.007	5	-8.483e-6	1	2740.72	2	NC	1
129		max	.004	1	.011	2	0	9	1.079e-3	4	NC	3	NC	1
130		min	-.005	3	-.011	3	-.007	5	-1.05e-5	1	3040.783	2	NC	1
131		max	.003	1	.01	2	0	9	1.1e-3	4	NC	3	NC	1
132		min	-.005	3	-.01	3	-.006	5	-1.252e-5	1	3405.319	2	NC	1
133		max	.003	1	.009	2	0	9	1.121e-3	4	NC	3	NC	1
134		min	-.004	3	-.009	3	-.006	5	-1.454e-5	1	3855.981	2	NC	1
135		max	.003	1	.008	2	0	9	1.141e-3	4	NC	3	NC	1
136		min	-.004	3	-.008	3	-.005	5	-1.655e-5	1	4425.265	2	NC	1
137		max	.002	1	.006	2	0	1	1.162e-3	4	NC	3	NC	1
138		min	-.003	3	-.007	3	-.005	5	-1.857e-5	1	5164.254	2	NC	1
139		max	.002	1	.005	2	0	1	1.183e-3	4	NC	3	NC	1
140		min	-.003	3	-.006	3	-.004	5	-2.059e-5	1	6158.105	2	NC	1
141		max	.002	1	.004	2	0	1	1.203e-3	4	NC	1	NC	1
142		min	-.002	3	-.005	3	-.003	5	-2.261e-5	1	7560.117	2	NC	1
143		max	.001	1	.003	2	0	1	1.224e-3	4	NC	1	NC	1
144		min	-.002	3	-.004	3	-.003	5	-2.463e-5	1	9676.909	2	NC	1
145		max	.001	1	.003	2	0	1	1.245e-3	4	NC	1	NC	1
146		min	-.001	3	-.003	3	-.002	5	-2.664e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.265e-3	4	NC	1	NC	1
148		min	0	3	-.002	3	-.001	5	-2.866e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.286e-3	4	NC	1	NC	1
150		min	0	3	-.001	3	0	5	-3.068e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.307e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.27e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.5e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.004e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.003	4	1.392e-5	1	NC	1	NC	1
156		min	0	2	-.001	3	0	1	-5.909e-4	4	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	1.285e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-5.813e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.01	4	1.178e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-5.718e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.013	4	1.07e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-5.622e-4	4	NC	1	NC	1
163		6	max	0	3	.005	2	.016	4	9.663e-6	3	NC	1	NC	1
164			min	0	2	-.007	3	0	1	-5.526e-4	4	9365.015	2	NC	1
165		7	max	0	3	.006	2	.019	4	2.745e-5	3	NC	1	NC	1
166			min	-.001	2	-.009	3	0	1	-5.431e-4	4	7760.414	2	NC	1
167		8	max	.001	3	.007	2	.022	4	4.524e-5	3	NC	3	NC	1
168			min	-.001	2	-.01	3	0	1	-5.335e-4	4	6574.976	2	NC	1
169		9	max	.001	3	.008	2	.025	4	6.303e-5	3	NC	3	NC	1
170			min	-.002	2	-.011	3	0	1	-5.24e-4	4	5658.038	2	NC	1
171		10	max	.001	3	.009	2	.028	4	8.082e-5	3	NC	3	NC	1
172			min	-.002	2	-.012	3	0	1	-5.144e-4	4	4926.134	2	NC	1
173		11	max	.001	3	.011	2	.031	4	9.86e-5	3	NC	3	NC	1
174			min	-.002	2	-.013	3	0	1	-5.049e-4	4	4328.992	2	NC	1
175		12	max	.002	3	.012	2	.033	4	1.164e-4	3	NC	3	NC	1
176			min	-.002	2	-.014	3	0	1	-4.953e-4	4	3834.196	2	NC	1
177		13	max	.002	3	.013	2	.036	4	1.342e-4	3	NC	3	NC	1
178			min	-.002	2	-.015	3	0	1	-4.857e-4	4	3419.616	2	NC	1
179		14	max	.002	3	.015	2	.038	4	1.52e-4	3	NC	3	NC	1
180			min	-.003	2	-.016	3	0	1	-4.762e-4	4	3069.41	2	NC	1
181		15	max	.002	3	.017	2	.041	4	1.698e-4	3	NC	3	NC	1
182			min	-.003	2	-.017	3	0	1	-4.666e-4	4	2771.797	2	NC	1
183		16	max	.002	3	.018	2	.043	4	1.875e-4	3	NC	3	NC	1
184			min	-.003	2	-.017	3	0	1	-4.571e-4	4	2517.75	2	NC	1
185		17	max	.002	3	.02	2	.045	4	2.053e-4	3	NC	3	NC	1
186			min	-.003	2	-.018	3	0	1	-4.475e-4	4	2300.194	2	NC	1
187		18	max	.002	3	.022	2	.047	4	2.231e-4	3	NC	3	NC	1
188			min	-.003	2	-.019	3	0	9	-4.379e-4	4	2113.493	2	NC	1
189		19	max	.003	3	.024	2	.05	4	2.409e-4	3	NC	3	NC	1
190			min	-.004	2	-.019	3	0	9	-4.284e-4	4	1953.11	2	NC	1
191	M8	1	max	.005	1	.022	2	0	9	2.234e-3	4	NC	1	NC	1
192			min	-.001	3	-.017	3	-.052	4	-1.835e-4	3	NC	1	372.23	4
193		2	max	.004	1	.021	2	0	9	2.234e-3	4	NC	1	NC	1
194			min	-.001	3	-.016	3	-.048	4	-1.835e-4	3	NC	1	405.738	4
195		3	max	.004	1	.02	2	0	9	2.234e-3	4	NC	1	NC	1
196			min	-.001	3	-.015	3	-.043	4	-1.835e-4	3	NC	1	445.615	4
197		4	max	.004	1	.019	2	0	9	2.234e-3	4	NC	1	NC	1
198			min	-.001	3	-.014	3	-.039	4	-1.835e-4	3	NC	1	493.537	4
199		5	max	.004	1	.017	2	0	9	2.234e-3	4	NC	1	NC	1
200			min	0	3	-.013	3	-.035	4	-1.835e-4	3	NC	1	551.789	4
201		6	max	.003	1	.016	2	0	9	2.234e-3	4	NC	1	NC	1
202			min	0	3	-.012	3	-.031	4	-1.835e-4	3	NC	1	623.548	4
203		7	max	.003	1	.015	2	0	9	2.234e-3	4	NC	1	NC	1
204			min	0	3	-.011	3	-.027	4	-1.835e-4	3	NC	1	713.338	4
205		8	max	.003	1	.014	2	0	9	2.234e-3	4	NC	1	NC	1
206			min	0	3	-.01	3	-.023	4	-1.835e-4	3	NC	1	827.784	4
207		9	max	.003	1	.012	2	0	9	2.234e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.02	4	-1.835e-4	3	NC	1	976.905	4
209		10	max	.002	1	.011	2	0	9	2.234e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.016	4	-1.835e-4	3	NC	1	1176.449	4
211		11	max	.002	1	.01	2	0	9	2.234e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.013	4	-1.835e-4	3	NC	1	1452.354	4
213		12	max	.002	1	.009	2	0	9	2.234e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214		min	0	3	-.007	3	-.01	4	-1.835e-4	3	NC	1	1849.858	4
215		max	.002	1	-.007	2	0	9	2.234e-3	4	NC	1	NC	1
216		min	0	3	-.006	3	-.008	4	-1.835e-4	3	NC	1	2453.813	4
217		max	.001	1	.006	2	0	9	2.234e-3	4	NC	1	NC	1
218		min	0	3	-.005	3	-.006	4	-1.835e-4	3	NC	1	3439.232	4
219		max	.001	1	.005	2	0	9	2.234e-3	4	NC	1	NC	1
220		min	0	3	-.004	3	-.004	4	-1.835e-4	3	NC	1	5217.259	4
221		max	0	1	.004	2	0	9	2.234e-3	4	NC	1	NC	1
222		min	0	3	-.003	3	-.002	4	-1.835e-4	3	NC	1	8956.62	4
223		max	0	1	.002	2	0	9	2.234e-3	4	NC	1	NC	1
224		min	0	3	-.002	3	-.001	4	-1.835e-4	3	NC	1	NC	1
225		max	0	1	.001	2	0	9	2.234e-3	4	NC	1	NC	1
226		min	0	3	0	3	0	4	-1.835e-4	3	NC	1	NC	1
227		max	0	1	0	1	0	1	2.234e-3	4	NC	1	NC	1
228		min	0	1	0	1	0	1	-1.835e-4	3	NC	1	NC	1
229	M10	max	.002	1	.007	2	0	3	2.594e-4	1	NC	3	NC	1
230		min	-.003	3	-.007	3	-.004	4	-4.056e-4	3	4462.798	2	NC	1
231		max	.002	1	.007	2	0	3	2.468e-4	1	NC	3	NC	1
232		min	-.002	3	-.006	3	-.004	4	-3.931e-4	3	4841.582	2	NC	1
233		max	.002	1	.006	2	0	3	2.426e-4	4	NC	3	NC	1
234		min	-.002	3	-.006	3	-.004	4	-3.806e-4	3	5287.121	2	NC	1
235		max	.002	1	.006	2	0	3	2.898e-4	4	NC	3	NC	1
236		min	-.002	3	-.006	3	-.004	4	-3.681e-4	3	5814.728	2	NC	1
237		max	.002	1	.005	2	0	3	3.37e-4	4	NC	1	NC	1
238		min	-.002	3	-.006	3	-.004	4	-3.556e-4	3	6444.582	2	NC	1
239		max	.001	1	.005	2	0	3	3.842e-4	4	NC	1	NC	1
240		min	-.002	3	-.005	3	-.004	4	-3.431e-4	3	7203.718	2	NC	1
241		max	.001	1	.004	2	0	3	4.314e-4	4	NC	1	NC	1
242		min	-.002	3	-.005	3	-.004	4	-3.306e-4	3	8129.027	2	NC	1
243		max	.001	1	.004	2	0	3	4.786e-4	4	NC	1	NC	1
244		min	-.002	3	-.005	3	-.004	4	-3.181e-4	3	9271.969	2	NC	1
245		max	.001	1	.003	2	0	3	5.258e-4	4	NC	1	NC	1
246		min	-.001	3	-.004	3	-.004	4	-3.056e-4	3	NC	1	NC	1
247		max	.001	1	.003	2	0	3	5.73e-4	4	NC	1	NC	1
248		min	-.001	3	-.004	3	-.004	4	-2.932e-4	3	NC	1	NC	1
249		max	0	1	.002	2	0	3	6.203e-4	4	NC	1	NC	1
250		min	-.001	3	-.004	3	-.003	4	-2.807e-4	3	NC	1	NC	1
251		max	0	1	.002	2	0	3	6.675e-4	4	NC	1	NC	1
252		min	-.001	3	-.003	3	-.003	4	-2.682e-4	3	NC	1	NC	1
253		max	0	1	.001	2	0	3	7.147e-4	4	NC	1	NC	1
254		min	0	3	-.003	3	-.003	4	-2.557e-4	3	NC	1	NC	1
255		max	0	1	.001	2	0	3	7.619e-4	4	NC	1	NC	1
256		min	0	3	-.003	3	-.002	4	-2.432e-4	3	NC	1	NC	1
257		max	0	1	0	2	0	3	8.091e-4	4	NC	1	NC	1
258		min	0	3	-.002	3	-.002	4	-2.307e-4	3	NC	1	NC	1
259		max	0	1	0	2	0	3	8.563e-4	4	NC	1	NC	1
260		min	0	3	-.002	3	-.002	4	-2.182e-4	3	NC	1	NC	1
261		max	0	1	0	2	0	3	9.035e-4	4	NC	1	NC	1
262		min	0	3	-.001	3	-.001	4	-2.057e-4	3	NC	1	NC	1
263		max	0	1	0	2	0	3	9.507e-4	4	NC	1	NC	1
264		min	0	3	0	3	0	4	-1.932e-4	3	NC	1	NC	1
265		max	0	1	0	1	0	1	9.979e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	-1.808e-4	3	NC	1	NC	1
267	M11	max	0	1	0	1	0	1	8.327e-5	3	NC	1	NC	1
268		min	0	1	0	1	0	1	-4.591e-4	4	NC	1	NC	1
269		max	0	3	0	2	.002	4	6.552e-5	3	NC	1	NC	1
270		min	0	2	0	3	0	3	-5.009e-4	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.01	5	7.046e-7	10	NC	1	2007.362	5
329		max	0	1	.003	2	0	1	2.898e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.007	5	7.046e-7	10	NC	1	2662.623	5
331		max	0	1	.002	2	0	1	2.898e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.005	5	7.046e-7	10	NC	1	3731.73	5
333		max	0	1	.002	2	0	1	2.898e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.003	5	7.046e-7	10	NC	1	5660.715	5
335		max	0	1	.001	2	0	1	2.898e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.002	5	7.046e-7	10	NC	1	9717.455	5
337		max	0	1	0	2	0	1	2.898e-3	4	NC	1	NC	1
338		min	0	3	0	3	0	5	7.046e-7	10	NC	1	NC	1
339		max	0	1	0	2	0	1	2.898e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	7.046e-7	10	NC	1	NC	1
341		max	0	1	0	1	0	1	2.898e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	7.046e-7	10	NC	1	NC	1
343	M1	max	.006	3	.024	3	.005	5	3.823e-3	1	NC	1	NC	1
344		min	-.007	2	-.021	2	0	9	-5.058e-3	3	NC	1	NC	1
345		max	.006	3	.013	3	.007	5	1.83e-3	1	NC	4	NC	1
346		min	-.007	2	-.011	2	-.002	1	-2.476e-3	3	4489.046	3	NC	1
347		max	.006	3	.003	3	.009	5	2.239e-4	5	NC	4	NC	1
348		min	-.007	2	-.002	1	-.003	1	-1.269e-4	1	2337.209	3	NC	1
349		max	.006	3	.006	2	.012	5	2.199e-4	5	NC	4	NC	1
350		min	-.007	2	-.005	3	-.003	1	-1.034e-4	1	1671.998	3	7417.098	5
351		max	.006	3	.013	2	.014	5	2.159e-4	5	NC	4	NC	1
352		min	-.007	2	-.012	3	-.003	1	-8.004e-5	1	1335.744	2	5240.176	5
353		max	.006	3	.019	2	.017	5	2.119e-4	5	NC	5	NC	1
354		min	-.007	2	-.017	3	-.003	1	-5.664e-5	1	1138.398	2	3985.258	5
355		max	.006	3	.023	2	.02	5	2.079e-4	5	NC	5	NC	1
356		min	-.007	2	-.021	3	-.003	1	-3.406e-5	9	1017.099	2	3181.931	5
357		max	.006	3	.027	2	.023	5	2.039e-4	5	NC	5	NC	1
358		min	-.007	2	-.024	3	-.002	1	-1.712e-5	9	941.512	2	2631.006	5
359		max	.006	3	.029	2	.026	5	2.016e-4	4	NC	5	NC	1
360		min	-.007	2	-.026	3	-.001	1	-1.801e-7	9	897.339	2	2233.828	4
361		max	.006	3	.03	2	.029	5	2.03e-4	4	NC	5	NC	1
362		min	-.007	2	-.026	3	0	9	4.712e-7	10	877.8	2	1922.611	4
363		max	.006	3	.029	2	.033	4	2.044e-4	4	NC	5	NC	1
364		min	-.007	2	-.025	3	0	9	5.538e-7	10	880.517	2	1687.026	4
365		max	.006	3	.027	2	.036	4	2.058e-4	4	NC	5	NC	1
366		min	-.007	2	-.023	3	0	10	6.365e-7	10	906.613	2	1504.749	4
367		max	.006	3	.024	2	.039	4	2.072e-4	4	NC	5	NC	1
368		min	-.007	2	-.02	3	0	10	7.191e-7	10	961.281	2	1361.369	4
369		max	.006	3	.019	2	.043	4	2.086e-4	4	NC	4	NC	1
370		min	-.007	2	-.016	3	0	10	8.018e-7	10	1056.372	2	1247.238	4
371		max	.006	3	.013	2	.046	4	2.1e-4	4	NC	4	NC	1
372		min	-.007	2	-.011	3	0	10	8.844e-7	10	1217.739	2	1155.714	4
373		max	.006	3	.005	2	.048	4	3.737e-4	4	NC	4	NC	1
374		min	-.007	2	-.004	3	0	10	9.521e-7	10	1508.116	2	1082.113	4
375		max	.006	3	.003	3	.051	4	4.401e-3	4	NC	4	NC	1
376		min	-.007	2	-.004	2	0	10	6.649e-7	10	2124.139	2	1023.156	4
377		max	.006	3	.01	3	.053	4	2.571e-3	4	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-1.268e-3	3	4094.051	2	976.264	4
379		max	.006	3	.019	3	.055	4	5.114e-3	2	NC	1	NC	1
380		min	-.007	2	-.027	2	0	1	-2.606e-3	3	NC	1	940.736	4
381	M5	max	.016	3	.062	3	.005	5	1.1e-5	4	NC	1	NC	1
382		min	-.019	2	-.054	2	0	9	0	1	NC	1	NC	1
383		max	.016	3	.035	3	.007	5	1.088e-4	5	NC	4	NC	1
384		min	-.019	2	-.03	2	0	9	-1.678e-5	9	1765.851	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.016	3	.009	3	.009	5	2.051e-4	5	NC	5	NC	1
386		min	-.019	2	-.007	1	0	9	-3.33e-5	9	914.557	3	NC	1
387	4	max	.016	3	.014	2	.012	5	2.131e-4	5	NC	5	NC	1
388		min	-.019	2	-.012	3	0	9	-3.147e-5	9	654.849	3	NC	1
389	5	max	.016	3	.032	2	.015	5	2.211e-4	5	NC	5	NC	1
390		min	-.019	2	-.029	3	0	9	-2.964e-5	9	523.548	2	NC	1
391	6	max	.016	3	.047	2	.018	5	2.29e-4	5	NC	5	NC	1
392		min	-.019	2	-.043	3	0	9	-2.782e-5	9	445.391	2	NC	1
393	7	max	.016	3	.059	2	.021	5	2.37e-4	5	NC	5	NC	1
394		min	-.019	2	-.054	3	0	9	-2.599e-5	9	397.281	2	NC	1
395	8	max	.016	3	.067	2	.024	5	2.449e-4	5	NC	5	NC	1
396		min	-.019	2	-.06	3	0	9	-2.416e-5	9	367.204	2	NC	1
397	9	max	.016	3	.073	2	.028	5	2.529e-4	5	NC	5	NC	1
398		min	-.019	2	-.064	3	0	9	-2.233e-5	9	349.493	2	NC	1
399	10	max	.016	3	.075	2	.031	5	2.609e-4	5	NC	5	NC	1
400		min	-.019	2	-.065	3	0	9	-2.05e-5	9	341.452	2	NC	1
401	11	max	.016	3	.074	2	.035	4	2.688e-4	5	NC	5	NC	1
402		min	-.019	2	-.062	3	0	9	-1.868e-5	9	342.119	2	NC	1
403	12	max	.016	3	.069	2	.038	4	2.768e-4	5	NC	5	NC	1
404		min	-.019	2	-.057	3	0	9	-1.685e-5	9	351.904	2	NC	1
405	13	max	.016	3	.061	2	.041	4	2.847e-4	5	NC	5	NC	1
406		min	-.019	2	-.049	3	0	9	-1.502e-5	9	372.808	2	NC	1
407	14	max	.016	3	.049	2	.044	4	2.927e-4	5	NC	5	NC	1
408		min	-.019	2	-.039	3	0	9	-1.319e-5	9	409.425	2	NC	1
409	15	max	.016	3	.033	2	.047	4	3.01e-4	4	NC	5	NC	1
410		min	-.019	2	-.026	3	0	9	-1.136e-5	9	471.816	2	NC	1
411	16	max	.016	3	.013	2	.05	4	4.691e-4	4	NC	5	NC	1
412		min	-.019	2	-.011	3	0	9	-1.061e-5	9	584.478	2	NC	1
413	17	max	.016	3	.007	3	.052	4	4.431e-3	4	NC	5	NC	1
414		min	-.02	2	-.011	2	0	9	-3.527e-5	9	824.879	2	NC	1
415	18	max	.016	3	.026	3	.054	4	2.275e-3	4	NC	4	NC	1
416		min	-.02	2	-.04	2	0	9	-1.805e-5	9	1600.43	2	NC	1
417	19	max	.016	3	.046	3	.055	4	4.364e-6	5	NC	1	NC	1
418		min	-.019	2	-.07	2	0	9	-5.731e-7	3	NC	1	NC	1
419	M9	1	max	.007	.023	3	.005	5	5.066e-3	3	NC	1	NC	1
420		min	-.007	2	-.021	2	0	9	-3.823e-3	1	NC	1	NC	1
421	2	max	.007	3	.013	3	.004	4	2.512e-3	3	NC	4	NC	1
422		min	-.007	2	-.011	2	0	10	-1.864e-3	1	4490.67	3	NC	1
423	3	max	.006	3	.003	3	.004	4	5.855e-5	1	NC	4	NC	1
424		min	-.007	2	-.002	1	0	3	-2.53e-5	5	2338.109	3	NC	1
425	4	max	.006	3	.006	2	.005	4	3.819e-5	1	NC	4	NC	1
426		min	-.007	2	-.005	3	-.001	3	-3.262e-5	5	1672.654	3	NC	1
427	5	max	.006	3	.013	2	.006	4	1.783e-5	1	NC	4	NC	1
428		min	-.007	2	-.012	3	-.002	3	-3.994e-5	5	1335.974	2	NC	1
429	6	max	.006	3	.019	2	.008	4	7.727e-6	2	NC	4	NC	1
430		min	-.007	2	-.018	3	-.003	3	-4.996e-5	4	1138.603	2	NC	1
431	7	max	.006	3	.023	2	.01	4	1.968e-6	2	NC	5	NC	1
432		min	-.007	2	-.022	3	-.003	3	-6.144e-5	4	1017.291	2	7634.932	4
433	8	max	.006	3	.027	2	.013	4	-1.713e-7	10	NC	5	NC	1
434		min	-.007	2	-.024	3	-.003	3	-7.293e-5	4	941.698	2	5241.876	4
435	9	max	.006	3	.029	2	.016	4	-2.619e-7	10	NC	5	NC	1
436		min	-.007	2	-.026	3	-.004	3	-8.441e-5	4	897.523	2	3872.563	4
437	10	max	.006	3	.03	2	.02	5	-3.525e-7	10	NC	5	NC	1
438		min	-.007	2	-.026	3	-.004	3	-9.59e-5	4	877.986	2	3011.626	4
439	11	max	.006	3	.029	2	.024	5	-4.431e-7	10	NC	5	NC	1
440		min	-.007	2	-.025	3	-.004	3	-1.074e-4	4	880.71	2	2433.381	4
441	12	max	.006	3	.027	2	.028	5	-5.337e-7	10	NC	5	NC	1









Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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