

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

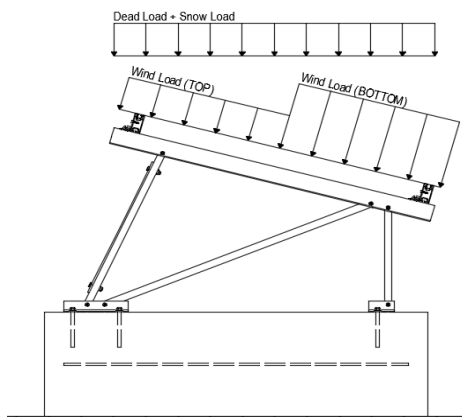
1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

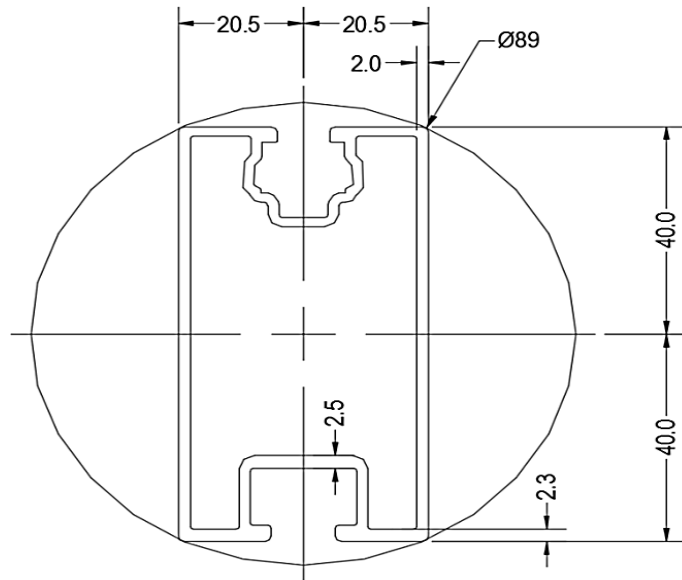
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

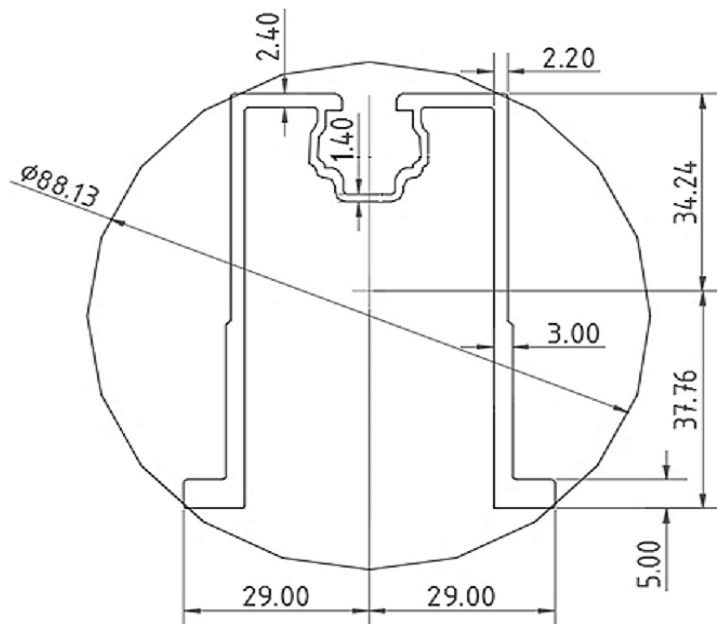
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	102 in
ΦF_{ty} STRONG-AXIS =	28.61 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.139 k-ft
M_z =	0.264 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	96%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.86 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.190 k-ft
M_z =	-0.151 k-ft
P_n =	0.063 k
$M_{y \text{ allowable}}$ =	1.466 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	43%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.059 k-ft
P_n =	0.306 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	16%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

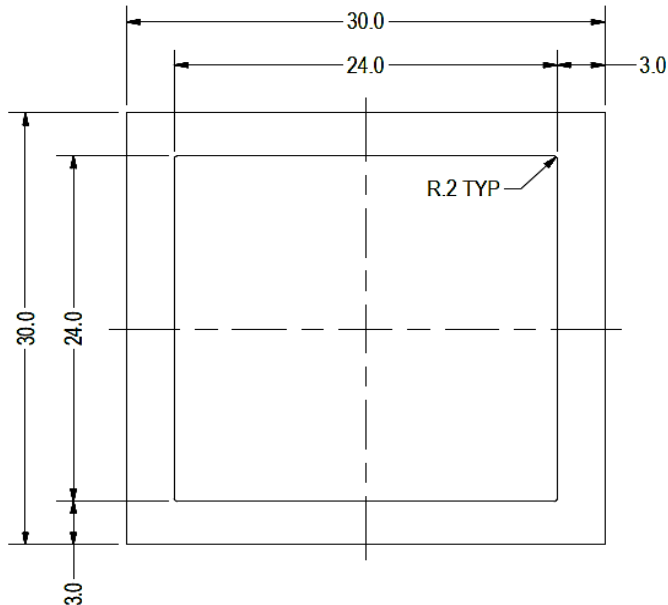
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.473 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

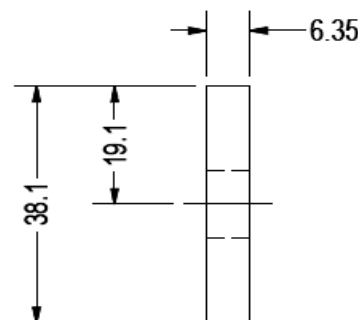
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.204 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<u>21%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.008 k-ft
P_n =	0.276 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>20%</u>



A cross brace kit is required every 10 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

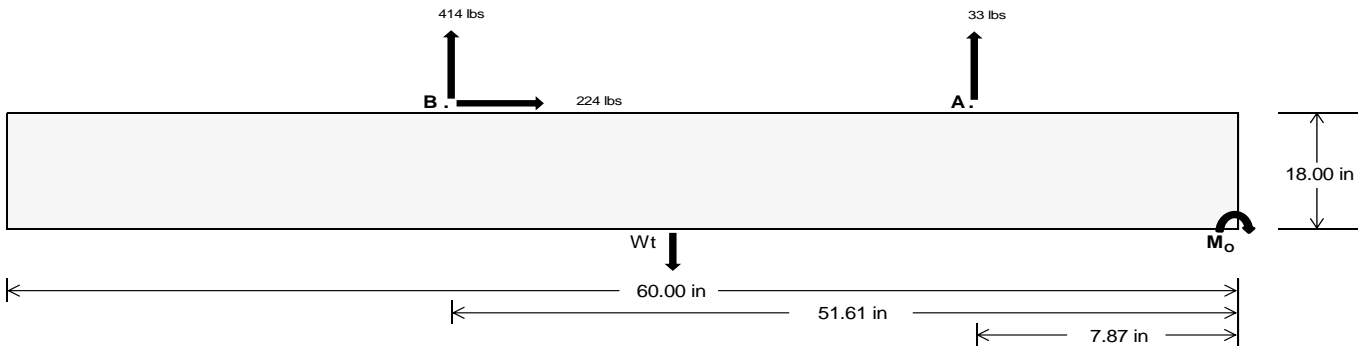
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>144.91</u>	<u>1725.77</u>	k
Compressive Load =	<u>1878.19</u>	<u>1544.65</u>	k
Lateral Load =	<u>47.76</u>	<u>930.46</u>	k
Moment (Weak Axis) =	<u>0.08</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 25649.2$ in-lbs
Resisting Force Required = 854.97 lbs
S.F. = 1.67
Weight Required = 1424.95 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 223.55 lbs
Friction = 0.4
Weight Required = 558.87 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 223.55 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	738 lbs	738 lbs	738 lbs	738 lbs	510 lbs	510 lbs	510 lbs	510 lbs	876 lbs	876 lbs	876 lbs	876 lbs	-67 lbs	-67 lbs	-67 lbs	-67 lbs
F_B	534 lbs	534 lbs	534 lbs	534 lbs	549 lbs	549 lbs	549 lbs	549 lbs	768 lbs	768 lbs	768 lbs	768 lbs	-828 lbs	-828 lbs	-828 lbs	-828 lbs
F_V	80 lbs	80 lbs	80 lbs	80 lbs	408 lbs	408 lbs	408 lbs	408 lbs	360 lbs	360 lbs	360 lbs	360 lbs	-447 lbs	-447 lbs	-447 lbs	-447 lbs
P_{total}	3266 lbs	3357 lbs	3448 lbs	3538 lbs	3053 lbs	3144 lbs	3234 lbs	3325 lbs	3638 lbs	3729 lbs	3819 lbs	3910 lbs	302 lbs	356 lbs	410 lbs	465 lbs
M	519 lbs-ft	519 lbs-ft	519 lbs-ft	519 lbs-ft	563 lbs-ft	563 lbs-ft	563 lbs-ft	563 lbs-ft	772 lbs-ft	772 lbs-ft	772 lbs-ft	772 lbs-ft	697 lbs-ft	697 lbs-ft	697 lbs-ft	697 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	0.21 ft	0.21 ft	0.20 ft	0.20 ft	2.31 ft	1.96 ft	1.70 ft	1.50 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	288.3 psf	285.3 psf	282.4 psf	279.8 psf	259.4 psf	257.6 psf	255.9 psf	254.4 psf	295.8 psf	292.4 psf	289.3 psf	286.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	424.3 psf	415.3 psf	407.1 psf	399.5 psf	406.7 psf	398.5 psf	391.0 psf	384.0 psf	498.0 psf	485.8 psf	474.6 psf	464.4 psf	580.1 psf	228.5 psf	170.7 psf	148.7 psf

Maximum Bearing Pressure = 580 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

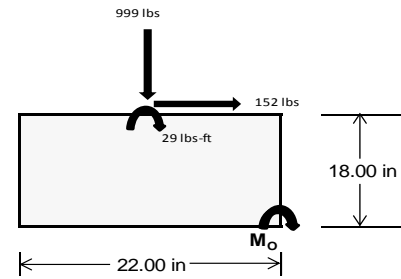
Overturning Check

$M_o = 658.1 \text{ ft-lbs}$
 Resisting Force Required = 717.97 lbs
 S.F. = 1.67
 Weight Required = 1196.61 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	151 lbs	199 lbs	95 lbs	394 lbs	999 lbs	350 lbs	83 lbs	16 lbs	31 lbs
F_v	25 lbs	201 lbs	26 lbs	16 lbs	152 lbs	20 lbs	25 lbs	201 lbs	26 lbs
P_{total}	2620 lbs	2667 lbs	2563 lbs	2744 lbs	3349 lbs	2700 lbs	805 lbs	738 lbs	752 lbs
M	72 lbs-ft	341 lbs-ft	78 lbs-ft	46 lbs-ft	258 lbs-ft	62 lbs-ft	73 lbs-ft	341 lbs-ft	78 lbs-ft
e	0.03 ft	0.13 ft	0.03 ft	0.02 ft	0.08 ft	0.02 ft	0.09 ft	0.46 ft	0.10 ft
$L/6$	0.31 ft	1.58 ft	1.77 ft	1.80 ft	1.68 ft	1.79 ft	1.65 ft	0.91 ft	1.63 ft
f_{min}	259.9 sqft	169.1 sqft	251.8 sqft	282.9 sqft	273.3 sqft	272.3 sqft	61.7 sqft	-41.2 sqft	54.3 sqft
f_{max}	311.6 psf	412.9 psf	307.4 psf	315.8 psf	457.3 psf	316.8 psf	114.0 psf	202.2 psf	109.8 psf



Maximum Bearing Pressure = 457 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

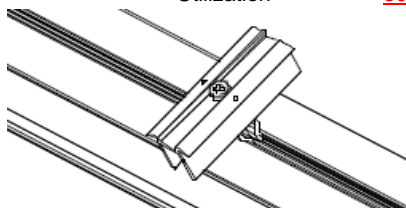
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

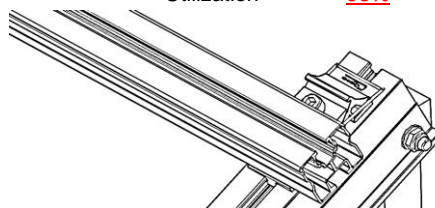
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.368 k
Allowable Uplift =	1.214 k
Utilization =	<u>30%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.099 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

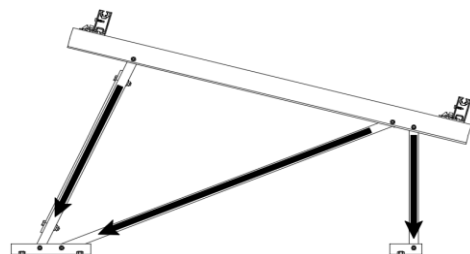
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.445 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

Diagonal Strut

Maximum Axial Load =	0.473 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.204 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.276 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.125 in
	<u>0.125 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.95$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 38.1$$

$$m = 0.63$$

$$C_0 = 40.784$$

$$Cc = 39.216$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 79.7$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.6 \text{ ksi}$$

$$I_x = 498305 \text{ mm}^4$$

$$1.197 \text{ in}^4$$

$$y = 40.784 \text{ mm}$$

$$S_x = 0.746 \text{ in}^3$$

$$M_{\max} St = 1.778 \text{ k-ft}$$

3.4.18

$$h/t = 6.6$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 20.5$$

$$Cc = 20.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 22.7 \text{ ksi}$$

$$I_y = 148662 \text{ mm}^4$$

$$0.357 \text{ in}^4$$

$$x = 20.5 \text{ mm}$$

$$S_y = 0.443 \text{ in}^3$$

$$M_{\max} Wk = 0.838 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 6.6$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

$$b/t = 37.95$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = (\phi k_2 \sqrt{(BpE)}) / (1.6b/t)$$

$$\phi F_L = 21.4 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 21.42 \text{ ksi}$$

$$A = 620.02 \text{ mm}^2$$

$$0.96 \text{ in}^2$$

$$P_{\max} = 20.59 \text{ kips}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.40 \\ &20.8038 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.40 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.466 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

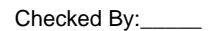
$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

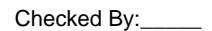
APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 25° 85mph 30psf 8.5ft 7-05.r3d Page 20



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 25° 85mph 30psf 8.5ft 7-05.r3D Page 21



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	368.275	1	.027	2	.807	1	0	12	.002	1	0	15
30			min	-363.293	3	-.03	3	-.341	5	-.001	1	0	3	0	6
31		16	max	368.391	1	-.008	10	.807	1	0	12	.002	1	0	15
32			min	-363.206	3	-.056	3	-.447	5	-.001	1	0	3	0	6
33		17	max	368.507	1	-.022	15	.807	1	0	12	.002	1	0	15
34			min	-363.118	3	-.093	4	-.552	5	-.001	1	0	3	0	6
35		18	max	368.624	1	-.033	15	.807	1	0	12	.002	1	0	15
36			min	-363.031	3	-.139	4	-.658	5	-.001	1	0	3	0	6
37		19	max	368.74	1	-.044	15	.807	1	0	12	.002	1	0	15
38			min	-362.944	3	-.184	4	-.763	5	-.001	1	0	3	0	6
39	M3	1	max	100.045	2	1.774	6	-.041	12	0	5	.003	1	0	6
40			min	-130.753	3	.417	15	-1.516	4	0	1	0	12	0	15
41		2	max	99.977	2	1.597	6	-.041	12	0	5	.002	1	0	2
42			min	-130.804	3	.375	15	-1.382	4	0	1	0	12	0	15
43		3	max	99.908	2	1.419	6	-.041	12	0	5	.002	1	0	2
44			min	-130.856	3	.333	15	-1.249	4	0	1	0	12	0	3
45		4	max	99.839	2	1.242	6	-.041	12	0	5	.002	1	0	15
46			min	-130.907	3	.292	15	-1.115	4	0	1	0	5	0	4
47		5	max	99.771	2	1.065	6	-.041	12	0	5	.002	1	0	15
48			min	-130.959	3	.25	15	-.981	4	0	1	0	5	0	4
49		6	max	99.702	2	.888	6	-.041	12	0	5	.002	1	0	15
50			min	-131.01	3	.208	15	-.856	1	0	1	0	5	0	4
51		7	max	99.634	2	.711	6	-.041	12	0	5	.002	1	0	15
52			min	-131.062	3	.167	15	-.856	1	0	1	0	5	0	4
53		8	max	99.565	2	.533	6	-.041	12	0	5	.001	1	0	15
54			min	-131.113	3	.125	15	-.856	1	0	1	0	5	-.001	4
55		9	max	99.496	2	.356	6	-.041	12	0	5	.001	1	0	15
56			min	-131.164	3	.083	15	-.856	1	0	1	0	5	-.001	4
57		10	max	99.428	2	.179	6	-.041	12	0	5	.001	1	0	15
58			min	-131.216	3	.042	15	-.856	1	0	1	0	5	-.001	4
59		11	max	99.359	2	.024	2	0	15	0	5	0	1	0	15
60			min	-131.267	3	-.022	3	-.856	1	0	1	0	5	-.001	4
61		12	max	99.291	2	-.042	15	.13	5	0	5	0	1	0	15
62			min	-131.319	3	-.176	4	-.856	1	0	1	0	5	-.001	4
63		13	max	99.222	2	-.083	15	.264	5	0	5	0	1	0	15
64			min	-131.37	3	-.353	4	-.856	1	0	1	0	5	-.001	4
65		14	max	99.153	2	-.125	15	.398	5	0	5	0	1	0	15
66			min	-131.422	3	-.53	4	-.856	1	0	1	0	5	-.001	4
67		15	max	99.085	2	-.167	15	.531	5	0	5	0	1	0	15
68			min	-131.473	3	-.707	4	-.856	1	0	1	0	5	0	4
69		16	max	99.016	2	-.208	15	.665	5	0	5	0	10	0	15
70			min	-131.525	3	-.884	4	-.856	1	0	1	0	4	0	4
71		17	max	98.948	2	-.25	15	.798	5	0	5	0	12	0	15
72			min	-131.576	3	-1.062	4	-.856	1	0	1	0	4	0	4
73		18	max	98.879	2	-.292	15	.932	5	0	5	0	15	0	15
74			min	-131.628	3	-1.239	4	-.856	1	0	1	0	1	0	4
75		19	max	98.81	2	-.333	15	1.066	5	0	5	0	5	0	1
76			min	-131.679	3	-1.416	4	-.856	1	0	1	0	1	0	1
77	M4	1	max	530.75	1	0	1	-.18	12	0	1	0	5	0	1
78			min	-24.687	3	0	1	-36.286	4	0	1	0	1	0	1
79		2	max	530.815	1	0	1	-.18	12	0	1	0	12	0	1
80			min	-24.638	3	0	1	-36.342	4	0	1	-.003	4	0	1
81		3	max	530.88	1	0	1	-.18	12	0	1	0	12	0	1
82			min	-24.59	3	0	1	-36.398	4	0	1	-.006	4	0	1
83		4	max	530.944	1	0	1	-.18	12	0	1	0	12	0	1
84			min	-24.541	3	0	1	-36.454	4	0	1	-.01	4	0	1
85		5	max	531.009	1	0	1	-.18	12	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-24.492	3	0	1	-36.51	4	0	1	-.013	4	0	1
87	6	max	531.074	1	0	1	-.18	12	0	1	0	12	0	1
88		min	-24.444	3	0	1	-36.566	4	0	1	-.016	4	0	1
89	7	max	531.139	1	0	1	-.18	12	0	1	0	12	0	1
90		min	-24.395	3	0	1	-36.622	4	0	1	-.02	4	0	1
91	8	max	531.203	1	0	1	-.18	12	0	1	0	12	0	1
92		min	-24.347	3	0	1	-36.678	4	0	1	-.023	4	0	1
93	9	max	531.268	1	0	1	-.18	12	0	1	0	12	0	1
94		min	-24.298	3	0	1	-36.734	4	0	1	-.026	4	0	1
95	10	max	531.333	1	0	1	-.18	12	0	1	0	12	0	1
96		min	-24.25	3	0	1	-36.79	4	0	1	-.029	4	0	1
97	11	max	531.397	1	0	1	-.18	12	0	1	0	12	0	1
98		min	-24.201	3	0	1	-36.847	4	0	1	-.033	4	0	1
99	12	max	531.462	1	0	1	-.18	12	0	1	0	12	0	1
100		min	-24.153	3	0	1	-36.903	4	0	1	-.036	4	0	1
101	13	max	531.527	1	0	1	-.18	12	0	1	0	12	0	1
102		min	-24.104	3	0	1	-36.959	4	0	1	-.039	4	0	1
103	14	max	531.591	1	0	1	-.18	12	0	1	0	12	0	1
104		min	-24.056	3	0	1	-37.015	4	0	1	-.043	4	0	1
105	15	max	531.656	1	0	1	-.18	12	0	1	0	12	0	1
106		min	-24.007	3	0	1	-37.071	4	0	1	-.046	4	0	1
107	16	max	531.721	1	0	1	-.18	12	0	1	0	12	0	1
108		min	-23.959	3	0	1	-37.127	4	0	1	-.049	4	0	1
109	17	max	531.786	1	0	1	-.18	12	0	1	0	12	0	1
110		min	-23.91	3	0	1	-37.183	4	0	1	-.053	4	0	1
111	18	max	531.85	1	0	1	-.18	12	0	1	0	12	0	1
112		min	-23.862	3	0	1	-37.239	4	0	1	-.056	4	0	1
113	19	max	531.915	1	0	1	-.18	12	0	1	0	12	0	1
114		min	-23.813	3	0	1	-37.295	4	0	1	-.059	4	0	1
115	M6	1	max	1202.234	1	.628	6	1.212	4	0	0	3	0	1
116		min	-1192.156	3	.142	15	-.112	3	0	5	0	2	0	1
117	2	max	1202.35	1	.582	6	1.107	4	0	1	0	4	0	15
118		min	-1192.069	3	.131	15	-.112	3	0	5	0	2	0	6
119	3	max	1202.467	1	.537	6	1.001	4	0	1	0	4	0	15
120		min	-1191.981	3	.121	15	-.112	3	0	5	0	2	0	6
121	4	max	1202.583	1	.491	6	.896	4	0	1	0	4	0	15
122		min	-1191.894	3	.11	15	-.112	3	0	5	0	12	0	6
123	5	max	1202.7	1	.449	2	.79	4	0	1	0	4	0	15
124		min	-1191.807	3	.099	15	-.112	3	0	5	0	12	0	6
125	6	max	1202.816	1	.414	2	.685	4	0	1	0	4	0	15
126		min	-1191.719	3	.088	15	-.112	3	0	5	0	3	0	6
127	7	max	1202.932	1	.378	2	.579	4	0	1	0	4	0	15
128		min	-1191.632	3	.078	15	-.112	3	0	5	0	3	0	6
129	8	max	1203.049	1	.343	2	.474	4	0	1	.001	4	0	15
130		min	-1191.545	3	.064	12	-.112	3	0	5	0	3	0	6
131	9	max	1203.165	1	.307	2	.368	4	0	1	.001	4	0	15
132		min	-1191.457	3	.046	12	-.112	3	0	5	0	3	0	2
133	10	max	1203.282	1	.271	2	.297	14	0	1	.001	4	0	15
134		min	-1191.37	3	.029	12	-.112	3	0	5	0	3	0	2
135	11	max	1203.398	1	.236	2	.244	14	0	1	.001	4	0	15
136		min	-1191.283	3	.01	3	-.112	3	0	5	0	3	0	2
137	12	max	1203.514	1	.2	2	.23	1	0	1	.001	4	0	15
138		min	-1191.196	3	-.017	3	-.112	3	0	5	0	3	0	2
139	13	max	1203.631	1	.165	2	.23	1	0	1	.001	4	0	15
140		min	-1191.108	3	-.043	3	-.148	5	0	5	0	3	0	2
141	14	max	1203.747	1	.129	2	.23	1	0	1	.001	4	0	15
142		min	-1191.021	3	-.07	3	-.254	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1203.864	1	.094	2	.23	1	0	1	.001	4	0	12
144		min	-1190.934	3	-.097	3	-.359	5	0	5	0	3	0	2
145	16	max	1203.98	1	.058	2	.23	1	0	1	.001	4	0	12
146		min	-1190.846	3	-.123	3	-.464	5	0	5	0	3	0	2
147	17	max	1204.096	1	.022	2	.23	1	0	1	.001	4	0	12
148		min	-1190.759	3	-.15	3	-.57	5	0	5	0	3	0	2
149	18	max	1204.213	1	-.013	2	.23	1	0	1	0	14	0	12
150		min	-1190.672	3	-.177	3	-.675	5	0	5	0	3	0	2
151	19	max	1204.329	1	-.049	2	.23	1	0	1	0	14	0	12
152		min	-1190.584	3	-.203	3	-.781	5	0	5	0	3	0	2
153	M7	1	max	472.906	2	1.788	.02	1	0	2	0	4	0	2
154		min	-404.916	3	.425	15	-1.394	5	0	3	0	3	0	12
155	2	max	472.837	2	1.611	4	.02	1	0	2	0	4	0	2
156		min	-404.968	3	.383	15	-1.261	5	0	3	0	3	0	3
157	3	max	472.769	2	1.434	4	.02	1	0	2	0	4	0	2
158		min	-405.019	3	.341	15	-1.127	5	0	3	0	3	0	3
159	4	max	472.7	2	1.256	4	.02	1	0	2	0	2	0	2
160		min	-405.071	3	.3	15	-.994	5	0	3	0	3	0	3
161	5	max	472.631	2	1.079	4	.02	1	0	2	0	2	0	15
162		min	-405.122	3	.258	15	-.86	5	0	3	0	5	0	3
163	6	max	472.563	2	.902	4	.02	1	0	2	0	2	0	15
164		min	-405.173	3	.217	15	-.726	5	0	3	0	5	0	6
165	7	max	472.494	2	.725	4	.02	1	0	2	0	2	0	15
166		min	-405.225	3	.175	15	-.593	5	0	3	0	5	0	6
167	8	max	472.426	2	.548	4	.02	1	0	2	0	2	0	15
168		min	-405.276	3	.133	15	-.459	5	0	3	0	5	-.001	6
169	9	max	472.357	2	.37	4	.02	1	0	2	0	2	0	15
170		min	-405.328	3	.086	12	-.325	5	0	3	0	5	-.001	6
171	10	max	472.288	2	.225	2	.02	1	0	2	0	2	0	15
172		min	-405.379	3	.017	12	-.192	5	0	3	0	5	-.001	6
173	11	max	472.22	2	.086	2	.02	1	0	2	0	2	0	15
174		min	-405.431	3	-.084	3	-.058	5	0	3	0	5	-.001	6
175	12	max	472.151	2	-.033	15	.08	4	0	2	0	2	0	15
176		min	-405.482	3	-.188	3	-.002	10	0	3	0	5	-.001	6
177	13	max	472.083	2	-.075	15	.214	4	0	2	0	2	0	15
178		min	-405.534	3	-.339	6	-.002	10	0	3	0	5	-.001	6
179	14	max	472.014	2	-.117	15	.348	4	0	2	0	2	0	15
180		min	-405.585	3	-.516	6	-.002	10	0	3	0	5	-.001	6
181	15	max	471.945	2	-.158	15	.481	4	0	2	0	2	0	15
182		min	-405.637	3	-.693	6	-.002	10	0	3	0	5	0	6
183	16	max	471.877	2	-.2	15	.615	4	0	2	0	2	0	15
184		min	-405.688	3	-.87	6	-.002	10	0	3	0	5	0	6
185	17	max	471.808	2	-.242	15	.748	4	0	2	0	2	0	15
186		min	-405.739	3	-1.048	6	-.002	10	0	3	0	5	0	6
187	18	max	471.74	2	-.283	15	.882	4	0	2	0	2	0	15
188		min	-405.791	3	-1.225	6	-.002	10	0	3	0	5	0	6
189	19	max	471.671	2	-.325	15	1.016	4	0	2	0	14	0	1
190		min	-405.842	3	-1.402	6	-.002	10	0	3	0	3	0	1
191	M8	1	max	1443.6	1	0	.919	1	0	1	0	4	0	1
192		min	-112.344	3	0	1	-36.296	4	0	1	0	1	0	1
193	2	max	1443.665	1	0	1	.919	1	0	1	0	1	0	1
194		min	-112.295	3	0	1	-36.352	4	0	1	-.003	4	0	1
195	3	max	1443.73	1	0	1	.919	1	0	1	0	1	0	1
196		min	-112.247	3	0	1	-36.408	4	0	1	-.006	4	0	1
197	4	max	1443.794	1	0	1	.919	1	0	1	0	1	0	1
198		min	-112.198	3	0	1	-36.464	4	0	1	-.01	4	0	1
199	5	max	1443.859	1	0	1	.919	1	0	1	0	1	0	1



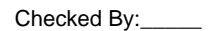
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-112.15	3	0	1	-36.52	4	0	1	-.013	4	0	1
201		6	max	1443.924	1	0	1	.919	1	0	1	0	1	0	1
202			min	-112.101	3	0	1	-36.576	4	0	1	-.016	4	0	1
203		7	max	1443.988	1	0	1	.919	1	0	1	0	1	0	1
204			min	-112.053	3	0	1	-36.632	4	0	1	-.02	4	0	1
205		8	max	1444.053	1	0	1	.919	1	0	1	0	1	0	1
206			min	-112.004	3	0	1	-36.688	4	0	1	-.023	4	0	1
207		9	max	1444.118	1	0	1	.919	1	0	1	0	1	0	1
208			min	-111.956	3	0	1	-36.745	4	0	1	-.026	4	0	1
209		10	max	1444.183	1	0	1	.919	1	0	1	0	1	0	1
210			min	-111.907	3	0	1	-36.801	4	0	1	-.029	4	0	1
211		11	max	1444.247	1	0	1	.919	1	0	1	0	1	0	1
212			min	-111.859	3	0	1	-36.857	4	0	1	-.033	4	0	1
213		12	max	1444.312	1	0	1	.919	1	0	1	0	1	0	1
214			min	-111.81	3	0	1	-36.913	4	0	1	-.036	4	0	1
215		13	max	1444.377	1	0	1	.919	1	0	1	0	1	0	1
216			min	-111.762	3	0	1	-36.969	4	0	1	-.039	4	0	1
217		14	max	1444.441	1	0	1	.919	1	0	1	.001	1	0	1
218			min	-111.713	3	0	1	-37.025	4	0	1	-.043	4	0	1
219		15	max	1444.506	1	0	1	.919	1	0	1	.001	1	0	1
220			min	-111.665	3	0	1	-37.081	4	0	1	-.046	4	0	1
221		16	max	1444.571	1	0	1	.919	1	0	1	.001	1	0	1
222			min	-111.616	3	0	1	-37.137	4	0	1	-.049	4	0	1
223		17	max	1444.635	1	0	1	.919	1	0	1	.001	1	0	1
224			min	-111.567	3	0	1	-37.193	4	0	1	-.053	4	0	1
225		18	max	1444.7	1	0	1	.919	1	0	1	.001	1	0	1
226			min	-111.519	3	0	1	-37.249	4	0	1	-.056	4	0	1
227		19	max	1444.765	1	0	1	.919	1	0	1	.001	1	0	1
228			min	-111.47	3	0	1	-37.305	4	0	1	-.059	4	0	1
229	M10	1	max	384.066	1	.664	4	1.413	5	.001	1	0	1	0	1
230			min	-350.321	3	.167	15	-.237	1	-.002	5	0	5	0	1
231		2	max	384.182	1	.619	4	1.307	5	.001	1	0	1	0	15
232			min	-350.234	3	.157	15	-.237	1	-.002	5	0	3	0	4
233		3	max	384.299	1	.573	4	1.202	5	.001	1	0	4	0	15
234			min	-350.147	3	.146	15	-.237	1	-.002	5	0	3	0	4
235		4	max	384.415	1	.527	4	1.096	5	.001	1	0	4	0	15
236			min	-350.059	3	.135	15	-.237	1	-.002	5	0	3	0	4
237		5	max	384.531	1	.482	4	.991	5	.001	1	0	4	0	15
238			min	-349.972	3	.124	15	-.237	1	-.002	5	0	3	0	4
239		6	max	384.648	1	.436	4	.885	5	.001	1	0	4	0	15
240			min	-349.885	3	.114	15	-.237	1	-.002	5	0	3	0	4
241		7	max	384.764	1	.39	4	.78	5	.001	1	.001	4	0	15
242			min	-349.797	3	.103	15	-.237	1	-.002	5	0	3	0	4
243		8	max	384.881	1	.345	4	.675	5	.001	1	.001	4	0	15
244			min	-349.71	3	.092	15	-.237	1	-.002	5	0	3	0	4
245		9	max	384.997	1	.299	4	.569	5	.001	1	.001	4	0	15
246			min	-349.623	3	.081	15	-.237	1	-.002	5	0	3	0	4
247		10	max	385.113	1	.253	4	.464	5	.001	1	.001	4	0	15
248			min	-349.535	3	.071	15	-.237	1	-.002	5	0	3	0	4
249		11	max	385.23	1	.208	4	.358	5	.001	1	.001	4	0	15
250			min	-349.448	3	.06	15	-.237	1	-.002	5	0	1	0	4
251		12	max	385.346	1	.162	4	.253	5	.001	1	.001	4	0	15
252			min	-349.361	3	.049	15	-.237	1	-.002	5	0	1	0	4
253		13	max	385.463	1	.116	4	.147	5	.001	1	.002	4	0	15
254			min	-349.274	3	.029	1	-.237	1	-.002	5	0	1	0	4
255		14	max	385.579	1	.071	4	.042	5	.001	1	.002	4	0	15
256			min	-349.186	3	-.007	1	-.237	1	-.002	5	0	1	0	4





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-23.987	3	0	1	-33.355	5	0	1	-.012	5	0	1
315	6	max	530.752	1	0	1	5.088	1	0	1	.002	1	0	1
316		min	-23.938	3	0	1	-33.411	5	0	1	-.015	5	0	1
317	7	max	530.816	1	0	1	5.088	1	0	1	.003	1	0	1
318		min	-23.89	3	0	1	-33.467	5	0	1	-.018	5	0	1
319	8	max	530.881	1	0	1	5.088	1	0	1	.003	1	0	1
320		min	-23.841	3	0	1	-33.523	5	0	1	-.021	5	0	1
321	9	max	530.946	1	0	1	5.088	1	0	1	.004	1	0	1
322		min	-23.793	3	0	1	-33.579	5	0	1	-.024	5	0	1
323	10	max	531.01	1	0	1	5.088	1	0	1	.004	1	0	1
324		min	-23.744	3	0	1	-33.635	5	0	1	-.027	5	0	1
325	11	max	531.075	1	0	1	5.088	1	0	1	.005	1	0	1
326		min	-23.696	3	0	1	-33.691	5	0	1	-.03	5	0	1
327	12	max	531.14	1	0	1	5.088	1	0	1	.005	1	0	1
328		min	-23.647	3	0	1	-33.747	5	0	1	-.033	5	0	1
329	13	max	531.204	1	0	1	5.088	1	0	1	.005	1	0	1
330		min	-23.599	3	0	1	-33.803	5	0	1	-.036	5	0	1
331	14	max	531.269	1	0	1	5.088	1	0	1	.006	1	0	1
332		min	-23.55	3	0	1	-33.859	5	0	1	-.039	5	0	1
333	15	max	531.334	1	0	1	5.088	1	0	1	.006	1	0	1
334		min	-23.502	3	0	1	-33.916	5	0	1	-.042	5	0	1
335	16	max	531.399	1	0	1	5.088	1	0	1	.007	1	0	1
336		min	-23.453	3	0	1	-33.972	5	0	1	-.045	5	0	1
337	17	max	531.463	1	0	1	5.088	1	0	1	.007	1	0	1
338		min	-23.405	3	0	1	-34.028	5	0	1	-.048	5	0	1
339	18	max	531.528	1	0	1	5.088	1	0	1	.008	1	0	1
340		min	-23.356	3	0	1	-34.084	5	0	1	-.051	5	0	1
341	19	max	531.593	1	0	1	5.088	1	0	1	.008	1	0	1
342		min	-23.308	3	0	1	-34.14	5	0	1	-.054	5	0	1
343	M1	1	max	161.948	1	342.178	3	-4.183	12	0	.197	1	.013	1
344		min	6.33	12	-363.767	1	-99.798	1	0	3	.009	12	-.01	3
345	2	max	162.066	1	341.988	3	-4.183	12	0	1	.176	1	.092	1
346		min	6.389	12	-364.02	1	-99.798	1	0	3	.008	12	-.084	3
347	3	max	111.573	1	7.892	9	-4.199	12	0	12	.152	1	.169	1
348		min	-1.269	10	-19.153	3	-99.666	1	0	1	.007	12	-.157	3
349	4	max	111.691	1	7.681	9	-4.199	12	0	12	.131	1	.169	1
350		min	-1.171	10	-19.343	3	-99.666	1	0	1	.006	12	-.153	3
351	5	max	111.809	1	7.47	9	-4.199	12	0	12	.109	1	.169	1
352		min	-1.072	10	-19.533	3	-99.666	1	0	1	.005	12	-.149	3
353	6	max	111.927	1	7.259	9	-4.199	12	0	12	.088	1	.17	1
354		min	-.974	10	-19.723	3	-99.666	1	0	1	.004	12	-.144	3
355	7	max	112.045	1	7.048	9	-4.199	12	0	12	.066	1	.17	1
356		min	-.876	10	-19.913	3	-99.666	1	0	1	.003	12	-.14	3
357	8	max	112.163	1	6.837	9	-4.199	12	0	12	.044	1	.17	1
358		min	-.777	10	-20.102	3	-99.666	1	0	1	.002	12	-.136	3
359	9	max	112.281	1	6.626	9	-4.199	12	0	12	.023	1	.171	1
360		min	-.679	10	-20.292	3	-99.666	1	0	1	.001	12	-.131	3
361	10	max	112.399	1	6.415	9	-4.199	12	0	12	.003	4	.171	1
362		min	-.581	10	-20.482	3	-99.666	1	0	1	0	10	-.127	3
363	11	max	112.517	1	6.204	9	-4.199	12	0	12	0	15	.172	1
364		min	-.482	10	-20.672	3	-99.666	1	0	1	-.02	1	-.122	3
365	12	max	112.635	1	5.994	9	-4.199	12	0	12	-.001	12	.173	2
366		min	-.384	10	-20.862	3	-99.666	1	0	1	-.042	1	-.118	3
367	13	max	112.753	1	5.783	9	-4.199	12	0	12	-.002	12	.177	2
368		min	-.286	10	-21.051	3	-99.666	1	0	1	-.064	1	-.113	3
369	14	max	112.871	1	5.572	9	-4.199	12	0	12	-.003	12	.182	2
370		min	-.187	10	-21.241	3	-99.666	1	0	1	-.085	1	-.109	3



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	112.989	1	5.361	9	-4.199	12	0	12	-.004	12	.186	2
372			min	-.089	10	-21.473	2	-99.666	1	0	1	-.107	1	-.104	3
373		16	max	90.393	2	55.23	2	-4.24	12	0	1	-.005	12	.19	2
374			min	-19.584	3	-125.039	3	-100.389	1	0	5	-.13	1	-.099	3
375		17	max	90.511	2	54.977	2	-4.24	12	0	1	-.006	12	.19	1
376			min	-19.496	3	-125.229	3	-100.389	1	0	5	-.151	1	-.071	3
377		18	max	-5.979	12	411.258	1	-4.42	12	0	5	-.007	12	.103	1
378			min	-161.332	1	-147.822	3	-102.958	1	0	1	-.174	1	-.039	3
379		19	max	-5.92	12	411.005	1	-4.42	12	0	5	-.008	12	.014	1
380			min	-161.214	1	-148.012	3	-102.958	1	0	1	-.196	1	-.007	3
381	M5	1	max	353.811	1	1131.257	3	-.128	10	0	1	.05	4	.02	3
382			min	10.063	15	-1203.993	1	-36.541	1	0	5	0	10	-.025	1
383		2	max	353.929	1	1131.067	3	-.128	10	0	1	.043	4	.236	1
384			min	10.099	15	-1204.246	1	-36.541	1	0	5	-.003	3	-.225	3
385		3	max	189.376	1	8.278	9	3.604	3	0	3	.036	4	.492	1
386			min	-20.852	10	-69.755	2	-26.798	4	0	4	-.01	3	-.466	3
387		4	max	189.494	1	8.067	9	3.604	3	0	3	.03	4	.498	1
388			min	-20.753	10	-70.008	2	-26.556	4	0	4	-.009	3	-.452	3
389		5	max	189.612	1	7.856	9	3.604	3	0	3	.025	4	.503	1
390			min	-20.655	10	-70.261	2	-26.314	4	0	4	-.008	3	-.438	3
391		6	max	189.73	1	7.645	9	3.604	3	0	3	.019	4	.509	1
392			min	-20.556	10	-70.515	2	-26.072	4	0	4	-.007	3	-.424	3
393		7	max	189.848	1	7.434	9	3.604	3	0	3	.013	4	.514	1
394			min	-20.458	10	-70.768	2	-25.83	4	0	4	-.007	1	-.41	3
395		8	max	189.966	1	7.223	9	3.604	3	0	3	.008	4	.52	1
396			min	-20.36	10	-71.021	2	-25.588	4	0	4	-.006	1	-.396	3
397		9	max	190.084	1	7.012	9	3.604	3	0	3	.002	5	.526	1
398			min	-20.261	10	-71.274	2	-25.346	4	0	4	-.006	1	-.382	3
399		10	max	190.202	1	6.802	9	3.604	3	0	3	0	10	.532	1
400			min	-20.163	10	-71.527	2	-25.104	4	0	4	-.005	1	-.368	3
401		11	max	190.32	1	6.591	9	3.604	3	0	3	0	10	.538	1
402			min	-20.065	10	-71.78	2	-24.862	4	0	4	-.009	4	-.353	3
403		12	max	190.438	1	6.38	9	3.604	3	0	3	0	10	.543	1
404			min	-19.966	10	-72.033	2	-24.62	4	0	4	-.014	4	-.339	3
405		13	max	190.556	1	6.169	9	3.604	3	0	3	0	10	.549	1
406			min	-19.868	10	-72.286	2	-24.378	4	0	4	-.019	4	-.325	3
407		14	max	190.674	1	5.958	9	3.604	3	0	3	0	10	.556	2
408			min	-19.77	10	-72.539	2	-24.136	4	0	4	-.025	4	-.311	3
409		15	max	190.792	1	5.747	9	3.604	3	0	3	0	10	.572	2
410			min	-19.671	10	-72.792	2	-23.894	4	0	4	-.03	4	-.296	3
411		16	max	314.739	2	299.12	2	3.58	3	0	1	0	3	.584	2
412			min	-65.843	3	-383.728	3	-22.653	4	0	4	-.035	4	-.279	3
413		17	max	314.857	2	298.867	2	3.58	3	0	1	0	3	.555	1
414			min	-65.754	3	-383.917	3	-22.411	4	0	4	-.04	4	-.196	3
415		18	max	-12.238	12	1354.338	1	3.332	1	0	4	.002	3	.266	1
416			min	-354.912	1	-486.333	3	-56.945	5	0	1	-.052	4	-.091	3
417		19	max	-12.179	12	1354.085	1	3.332	1	0	4	.002	3	.015	3
418			min	-354.794	1	-486.523	3	-56.703	5	0	1	-.064	4	-.027	1
419	M9	1	max	161.175	1	342.162	3	241.57	4	0	3	-.003	15	.013	1
420			min	4	15	-363.743	1	9.598	10	0	1	-.197	1	-.01	3
421		2	max	161.293	1	341.972	3	241.812	4	0	3	.043	5	.092	1
422			min	4.036	15	-363.996	1	9.598	10	0	1	-.167	1	-.084	3
423		3	max	111.474	1	7.863	9	93.642	1	0	1	.088	5	.169	1
424			min	-.723	10	-19.095	3	-33.738	5	0	5	-.136	1	-.157	3
425		4	max	111.592	1	7.652	9	93.642	1	0	1	.08	5	.169	1
426			min	-.625	10	-19.284	3	-33.496	5	0	5	-.116	1	-.153	3
427		5	max	111.71	1	7.441	9	93.642	1	0	1	.073	5	.169	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-526	10	-19.474	3	-33.254	5	0	5	-.096	1	-.149	3
429	6	max	111.828	1	7.23	9	93.642	1	0	1	.066	5	.17	1
430		min	-.428	10	-19.664	3	-33.012	5	0	5	-.075	1	-.144	3
431	7	max	111.946	1	7.019	9	93.642	1	0	1	.059	5	.17	1
432		min	-.33	10	-19.854	3	-32.77	5	0	5	-.055	1	-.14	3
433	8	max	112.064	1	6.809	9	93.642	1	0	1	.052	5	.17	1
434		min	-.231	10	-20.044	3	-32.528	5	0	5	-.035	1	-.136	3
435	9	max	112.182	1	6.598	9	93.642	1	0	1	.045	5	.171	1
436		min	-.133	10	-20.233	3	-32.286	5	0	5	-.015	1	-.131	3
437	10	max	112.3	1	6.387	9	93.642	1	0	1	.038	4	.171	1
438		min	-.035	10	-20.423	3	-32.044	5	0	5	0	10	-.127	3
439	11	max	112.418	1	6.176	9	93.642	1	0	1	.035	4	.172	1
440		min	.064	10	-20.613	3	-31.802	5	0	5	.002	10	-.122	3
441	12	max	112.536	1	5.965	9	93.642	1	0	1	.046	1	.173	2
442		min	.162	10	-20.803	3	-31.56	5	0	5	.004	10	-.118	3
443	13	max	112.654	1	5.754	9	93.642	1	0	1	.067	1	.177	2
444		min	.26	10	-20.993	3	-31.318	5	0	5	.005	12	-.113	3
445	14	max	112.772	1	5.543	9	93.642	1	0	1	.087	1	.182	2
446		min	.359	10	-21.235	2	-31.076	5	0	5	.005	12	-.109	3
447	15	max	112.89	1	5.332	9	93.642	1	0	1	.107	1	.186	2
448		min	.457	10	-21.488	2	-30.834	5	0	5	.002	15	-.104	3
449	16	max	90.667	2	55.062	2	94.492	1	0	10	.129	1	.19	2
450		min	-19.599	3	-125.496	3	-29.359	5	0	4	0	5	-.099	3
451	17	max	90.785	2	54.809	2	94.492	1	0	10	.15	1	.19	1
452		min	-19.51	3	-125.685	3	-29.117	5	0	4	-.007	5	-.071	3
453	18	max	.335	15	411.258	1	99.647	1	0	1	.172	1	.103	1
454		min	-161.044	1	-147.82	3	-61.886	5	0	3	-.02	5	-.039	3
455	19	max	.37	15	411.004	1	99.647	1	0	1	.193	1	.014	1
456		min	-160.926	1	-148.01	3	-61.644	5	0	3	-.034	5	-.007	3
457	M13	1	max	241.591	4	363.063	1	-4	.013	1	.197	1	0	1
458		min	9.602	10	-342.143	3	-161.153	1	-.01	3	.003	15	0	3
459	2	max	232.239	4	256.121	1	-2.624	15	.013	1	.063	1	.275	3
460		min	9.602	10	-241.292	3	-123.546	1	-.01	3	0	15	-.292	1
461	3	max	222.888	4	149.179	1	-1.249	15	.013	1	.001	3	.456	3
462		min	9.602	10	-140.441	3	-85.938	1	-.01	3	-.036	1	-.484	1
463	4	max	213.536	4	42.237	1	.127	15	.013	1	-.002	12	.541	3
464		min	9.602	10	-39.589	3	-48.331	1	-.01	3	-.1	1	-.574	1
465	5	max	204.185	4	61.262	3	2.218	5	.013	1	-.002	15	.531	3
466		min	9.602	10	-64.705	1	-10.723	1	-.01	3	-.128	1	-.564	1
467	6	max	194.833	4	162.113	3	26.884	1	.013	1	0	15	.425	3
468		min	9.602	10	-171.647	1	.5	12	-.01	3	-.12	1	-.452	1
469	7	max	185.482	4	262.965	3	64.492	1	.013	1	.005	5	.224	3
470		min	9.602	10	-278.589	1	1.842	12	-.01	3	-.077	1	-.239	1
471	8	max	176.13	4	363.816	3	102.099	1	.013	1	.013	4	.074	1
472		min	9.602	10	-385.531	1	3.184	12	-.01	3	0	3	-.072	3
473	9	max	166.779	4	464.667	3	139.707	1	.013	1	.116	1	.489	1
474		min	9.602	10	-492.474	1	4.527	12	-.01	3	.004	12	-.463	3
475	10	max	157.427	4	565.518	3	177.314	1	.011	2	.266	1	1.005	1
476		min	9.602	10	-599.416	1	5.869	12	-.013	1	.008	12	-.949	3
477	11	max	117.497	4	492.473	1	-.308	15	.01	3	.11	1	.489	1
478		min	4.184	12	-464.667	3	-138.928	1	-.013	1	-.017	5	-.463	3
479	12	max	108.145	4	385.531	1	1.371	5	.01	3	0	10	.074	1
480		min	4.184	12	-363.816	3	-101.321	1	-.013	1	-.017	4	-.072	3
481	13	max	100.191	1	278.589	1	3.499	5	.01	3	-.004	12	.224	3
482		min	4.184	12	-262.964	3	-63.713	1	-.013	1	-.081	1	-.239	1
483	14	max	100.191	1	171.647	1	5.627	5	.01	3	-.005	12	.425	3
484		min	4.184	12	-162.113	3	-26.106	1	-.013	1	-.123	1	-.452	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	100.191	1	64.705	1	11.502	1	.01	3	-.002	15	.531	3
486			min	4.184	12	-61.262	3	.795	10	-.013	1	-.13	1	-.564	1
487		16	max	100.191	1	39.589	3	49.109	1	.01	3	.005	5	.541	3
488			min	4.184	12	-42.237	1	2.304	12	-.013	1	-.102	1	-.574	1
489		17	max	100.191	1	140.441	3	86.717	1	.01	3	.015	5	.456	3
490			min	4.184	12	-149.179	1	3.646	12	-.013	1	-.038	1	-.484	1
491		18	max	100.191	1	241.292	3	124.324	1	.01	3	.062	1	.275	3
492			min	4.184	12	-256.121	1	4.988	12	-.013	1	.003	12	-.292	1
493		19	max	100.191	1	342.143	3	161.932	1	.01	3	.197	1	0	1
494			min	4.184	12	-363.064	1	6.331	12	-.013	1	.009	12	0	3
495	M16	1	max	61.643	5	411.732	1	.37	15	.007	3	.193	1	0	1
496			min	-99.216	1	-148.036	3	-160.943	1	-.014	1	-.034	5	0	3
497		2	max	52.292	5	290.449	1	2.327	5	.007	3	.059	1	.119	3
498			min	-99.216	1	-104.55	3	-123.335	1	-.014	1	-.033	5	-.332	1
499		3	max	42.94	5	169.166	1	4.455	5	.007	3	-.001	12	.197	3
500			min	-99.216	1	-61.063	3	-85.728	1	-.014	1	-.04	1	-.549	1
501		4	max	33.589	5	47.882	1	6.583	5	.007	3	-.004	12	.235	3
502			min	-99.216	1	-17.577	3	-48.12	1	-.014	1	-.103	1	-.651	1
503		5	max	24.237	5	25.91	3	8.711	5	.007	3	-.005	12	.231	3
504			min	-99.216	1	-73.401	1	-10.513	1	-.014	1	-.131	1	-.639	1
505		6	max	14.886	5	69.396	3	27.095	1	.007	3	-.005	15	.186	3
506			min	-99.216	1	-194.684	1	.65	12	-.014	1	-.123	1	-.512	1
507		7	max	5.534	5	112.883	3	64.702	1	.007	3	.004	5	.1	3
508			min	-99.216	1	-315.968	1	1.992	12	-.014	1	-.08	1	-.271	1
509		8	max	-2.348	12	156.369	3	102.31	1	.007	3	.017	4	.084	1
510			min	-99.216	1	-437.251	1	3.334	12	-.014	1	-.002	3	-.028	3
511		9	max	-2.348	12	199.856	3	139.917	1	.007	3	.114	1	.555	1
512			min	-99.216	1	-558.534	1	4.676	12	-.014	1	.002	12	-.196	3
513		10	max	34.22	5	-16.532	15	177.525	1	.005	14	.264	1	1.139	1
514			min	-102.583	1	-679.818	1	-9.226	3	-.014	1	.009	12	-.405	3
515		11	max	24.869	5	558.534	1	-.254	15	.014	1	.114	1	.555	1
516			min	-102.583	1	-199.856	3	-139.628	1	-.007	3	-.015	5	-.196	3
517		12	max	15.517	5	437.251	1	1.448	5	.014	1	0	2	.084	1
518			min	-102.583	1	-156.369	3	-102.021	1	-.007	3	-.015	4	-.028	3
519		13	max	6.166	5	315.968	1	3.576	5	.014	1	-.003	12	.1	3
520			min	-102.583	1	-112.883	3	-64.413	1	-.007	3	-.078	1	-.271	1
521		14	max	-1.987	15	194.684	1	5.704	5	.014	1	-.004	12	.186	3
522			min	-102.583	1	-69.396	3	-26.806	1	-.007	3	-.121	1	-.512	1
523		15	max	-4.42	12	73.401	1	10.801	1	.014	1	0	15	.231	3
524			min	-102.583	1	-25.91	3	.551	12	-.007	3	-.129	1	-.639	1
525		16	max	-4.42	12	17.577	3	48.409	1	.014	1	.007	5	.235	3
526			min	-102.583	1	-47.882	1	1.893	12	-.007	3	-.101	1	-.651	1
527		17	max	-4.42	12	61.063	3	86.016	1	.014	1	.017	5	.197	3
528			min	-102.583	1	-169.166	1	3.235	12	-.007	3	-.038	1	-.549	1
529		18	max	-4.42	12	104.55	3	123.624	1	.014	1	.061	1	.119	3
530			min	-102.583	1	-290.449	1	4.577	12	-.007	3	.003	12	-.332	1
531		19	max	-4.42	12	148.036	3	161.231	1	.014	1	.196	1	0	1
532			min	-102.583	1	-411.732	1	5.919	12	-.007	3	.008	12	0	5
533	M15	1	max	0	4	2.307	2	.024	3	0	1	0	1	0	1
534			min	-40.623	1	0	4	-.027	1	0	3	0	3	0	1
535		2	max	0	4	2.051	2	.024	3	0	1	0	1	0	4
536			min	-40.71	1	0	4	-.027	1	0	3	0	3	-.001	2
537		3	max	0	4	1.795	2	.024	3	0	1	0	1	0	4
538			min	-40.797	1	0	4	-.027	1	0	3	0	3	-.002	2
539		4	max	0	4	1.538	2	.024	3	0	1	0	1	0	4
540			min	-40.884	1	0	4	-.027	1	0	3	0	3	-.003	2
541		5	max	0	4	1.282	2	.024	3	0	1	0	1	0	4



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-40.971	1	0	4	-.027	1	0	3	0	3	-.004	2
543		6	max	0	4	1.026	2	.024	3	0	1	0	1	0	4
544			min	-41.058	1	0	4	-.027	1	0	3	0	3	-.004	2
545		7	max	0	4	.769	2	.024	3	0	1	0	3	0	4
546			min	-41.145	1	0	4	-.027	1	0	3	0	2	-.005	2
547		8	max	0	4	.513	2	.024	3	0	1	0	3	0	4
548			min	-41.232	1	0	4	-.027	1	0	3	0	1	-.005	2
549		9	max	0	4	.256	2	.024	3	0	1	0	3	0	4
550			min	-41.319	1	0	4	-.027	1	0	3	0	1	-.005	2
551		10	max	0	4	0	1	.024	3	0	1	0	3	0	4
552			min	-41.406	1	0	1	-.027	1	0	3	0	1	-.005	2
553		11	max	0	4	0	4	.024	3	0	1	0	3	0	4
554			min	-41.493	1	-.256	1	-.027	1	0	3	0	1	-.005	2
555		12	max	0	4	0	4	.024	3	0	1	0	3	0	4
556			min	-41.579	1	-.513	1	-.027	1	0	3	0	1	-.005	2
557		13	max	0	4	0	4	.024	3	0	1	0	3	0	4
558			min	-41.666	1	-.769	1	-.027	1	0	3	0	1	-.005	2
559		14	max	0	4	0	4	.024	3	0	1	0	3	0	4
560			min	-41.753	1	-1.026	1	-.027	1	0	3	0	1	-.004	2
561		15	max	0	4	0	4	.024	3	0	1	0	3	0	4
562			min	-41.84	1	-1.282	1	-.027	1	0	3	0	1	-.004	2
563		16	max	0	4	0	4	.024	3	0	1	0	3	0	4
564			min	-41.927	1	-1.538	1	-.027	1	0	3	0	1	-.003	2
565		17	max	0	4	0	4	.024	3	0	1	0	3	0	4
566			min	-42.014	1	-1.795	1	-.027	1	0	3	0	1	-.002	2
567		18	max	0	4	0	4	.024	3	0	1	0	3	0	4
568			min	-42.101	1	-2.051	1	-.027	1	0	3	0	1	-.001	2
569		19	max	0	4	0	4	.024	3	0	1	0	3	0	1
570			min	-42.188	1	-2.307	1	-.027	1	0	3	0	1	0	1
571	M16A	1	max	-1.024	10	3.636	4	.247	4	0	3	0	3	0	1
572			min	-274.253	4	1.15	15	-.01	3	0	1	0	4	0	1
573		2	max	-.952	10	3.232	4	.223	4	0	3	0	3	0	15
574			min	-274.362	4	1.022	15	-.01	3	0	1	0	4	-.002	4
575		3	max	-.88	10	2.828	4	.198	4	0	3	0	3	-.001	15
576			min	-274.472	4	.894	15	-.01	3	0	1	0	4	-.003	4
577		4	max	-.807	10	2.424	4	.174	4	0	3	0	3	-.001	15
578			min	-274.581	4	.767	15	-.01	3	0	1	0	4	-.005	4
579		5	max	-.735	10	2.02	4	.15	4	0	3	0	3	-.002	15
580			min	-274.691	4	.639	15	-.01	3	0	1	0	1	-.006	4
581		6	max	-.662	10	1.616	4	.126	4	0	3	0	5	-.002	15
582			min	-274.8	4	.511	15	-.01	3	0	1	0	1	-.007	4
583		7	max	-.59	10	1.212	4	.101	4	0	3	0	5	-.002	15
584			min	-274.909	4	.383	15	-.01	3	0	1	0	1	-.007	4
585		8	max	-.517	10	.808	4	.077	4	0	3	0	5	-.002	15
586			min	-275.019	4	.256	15	-.01	3	0	1	0	1	-.008	4
587		9	max	-.445	10	.404	4	.053	4	0	3	0	5	-.003	15
588			min	-275.128	4	.128	15	-.01	3	0	1	0	1	-.008	4
589		10	max	-.372	10	0	1	.028	4	0	3	0	5	-.003	15
590			min	-275.238	4	0	1	-.01	3	0	1	0	1	-.008	4
591		11	max	-.3	10	-.128	15	.02	1	0	3	0	5	-.003	15
592			min	-275.347	4	-.404	4	-.01	3	0	1	0	1	-.008	4
593		12	max	-.228	10	-.256	15	.02	1	0	3	0	5	-.002	15
594			min	-275.457	4	-.808	4	-.024	5	0	1	0	1	-.008	4
595		13	max	-.155	10	-.383	15	.02	1	0	3	0	5	-.002	15
596			min	-275.566	4	-1.212	4	-.049	5	0	1	0	3	-.007	4
597		14	max	-.083	10	-.511	15	.02	1	0	3	0	4	-.002	15
598			min	-275.676	4	-1.616	4	-.073	5	0	1	0	3	-.007	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	-.01	10	-.639	15	.02	1	0	3	0	4	-.002	15
600		min	-275.785	4	-2.02	4	-.097	5	0	1	0	3	-.006	4
601	16	max	.062	10	-.767	15	.02	1	0	3	0	4	-.001	15
602		min	-275.895	4	-2.424	4	-.122	5	0	1	0	3	-.005	4
603	17	max	.135	10	-.894	15	.02	1	0	3	0	1	-.001	15
604		min	-276.004	4	-2.828	4	-.146	5	0	1	0	5	-.003	4
605	18	max	.207	10	-1.022	15	.02	1	0	3	0	1	0	15
606		min	-276.113	4	-3.232	4	-.17	5	0	1	0	5	-.002	4
607	19	max	.279	10	-1.15	15	.02	1	0	3	0	1	0	1
608		min	-276.223	4	-3.636	4	-.194	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.003	1	.009	2	.018	1	2.066e-3	5	NC	3	NC	3
2			min	-.003	3	-.008	3	-.02	5	-1.579e-3	1	4166.282	2	1970.15	1
3		2	max	.003	1	.008	2	.017	1	2.093e-3	5	NC	3	NC	3
4			min	-.003	3	-.008	3	-.02	5	-1.512e-3	1	4526.106	2	2126.925	1
5		3	max	.003	1	.007	2	.016	1	2.12e-3	5	NC	3	NC	3
6			min	-.003	3	-.007	3	-.019	5	-1.445e-3	1	4950.335	2	2311.75	1
7		4	max	.003	1	.007	2	.014	1	2.147e-3	5	NC	3	NC	3
8			min	-.003	3	-.007	3	-.018	5	-1.378e-3	1	5453.797	2	2531.371	1
9		5	max	.003	1	.006	2	.013	1	2.174e-3	5	NC	3	NC	3
10			min	-.003	3	-.007	3	-.017	5	-1.311e-3	1	6056.053	2	2794.784	1
11		6	max	.002	1	.005	2	.012	1	2.202e-3	5	NC	1	NC	3
12			min	-.002	3	-.006	3	-.016	5	-1.244e-3	1	6783.314	2	3114.213	1
13		7	max	.002	1	.005	2	.01	1	2.229e-3	5	NC	1	NC	3
14			min	-.002	3	-.006	3	-.015	5	-1.177e-3	1	7671.363	2	3506.601	1
15		8	max	.002	1	.004	2	.009	1	2.256e-3	5	NC	1	NC	3
16			min	-.002	3	-.006	3	-.014	5	-1.11e-3	1	8770.114	2	3996.009	1
17		9	max	.002	1	.004	2	.008	1	2.283e-3	5	NC	1	NC	2
18			min	-.002	3	-.005	3	-.013	5	-1.043e-3	1	NC	1	4617.561	1
19		10	max	.002	1	.003	2	.007	1	2.31e-3	5	NC	1	NC	2
20			min	-.002	3	-.005	3	-.012	5	-9.763e-4	1	NC	1	5424.246	1
21		11	max	.001	1	.003	2	.006	1	2.338e-3	5	NC	1	NC	2
22			min	-.001	3	-.004	3	-.011	5	-9.094e-4	1	NC	1	6499.22	1
23		12	max	.001	1	.002	2	.005	1	2.365e-3	5	NC	1	NC	2
24			min	-.001	3	-.004	3	-.01	5	-8.425e-4	1	NC	1	7979.413	1
25		13	max	.001	1	.002	2	.004	1	2.392e-3	5	NC	1	NC	1
26			min	-.001	3	-.003	3	-.009	5	-7.755e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	.003	1	2.419e-3	5	NC	1	NC	1
28			min	0	3	-.003	3	-.007	5	-7.086e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	.002	1	2.446e-3	5	NC	1	NC	1
30			min	0	3	-.002	3	-.006	5	-6.417e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	.001	1	2.474e-3	5	NC	1	NC	1
32			min	0	3	-.002	3	-.005	5	-5.748e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	2.501e-3	5	NC	1	NC	1
34			min	0	3	-.001	3	-.003	5	-5.078e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	2.528e-3	5	NC	1	NC	1
36			min	0	3	0	3	-.002	5	-4.409e-4	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	2.555e-3	5	NC	1	NC	1
38			min	0	1	0	1	0	1	-3.74e-4	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	1.741e-4	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-1.189e-3	5	NC	1	NC	1
41		2	max	0	3	0	2	.006	5	2.173e-4	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-1.2e-3	5	NC	1	NC	1



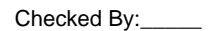
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43	3	max	0	3	0	2	.012	5	2.605e-4	1	NC	1	NC	1
44		min	0	2	-.002	3	-.001	1	-1.21e-3	5	NC	1	8092.245	14
45	4	max	0	3	0	2	.019	5	3.037e-4	1	NC	1	NC	1
46		min	0	2	-.002	3	-.002	1	-1.221e-3	5	NC	1	5268.988	14
47	5	max	0	3	0	2	.025	5	3.469e-4	1	NC	1	NC	1
48		min	0	2	-.003	3	-.002	1	-1.231e-3	5	NC	1	3870.542	14
49	6	max	0	3	0	2	.031	4	3.901e-4	1	NC	1	NC	1
50		min	0	2	-.004	3	-.001	1	-1.242e-3	5	NC	1	3040.576	14
51	7	max	0	3	0	2	.037	4	4.333e-4	1	NC	1	NC	1
52		min	0	2	-.005	3	-.001	1	-1.253e-3	5	NC	1	2493.857	14
53	8	max	0	3	.001	2	.044	4	4.765e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	-1.263e-3	5	NC	1	2108.281	14
55	9	max	0	3	.002	2	.05	4	5.196e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	2	-1.274e-3	5	NC	1	1822.893	14
57	10	max	0	3	.002	2	.056	4	5.628e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	10	-1.285e-3	5	NC	1	1603.898	14
59	11	max	0	3	.003	2	.062	4	6.06e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	10	-1.295e-3	5	NC	1	1431.064	14
61	12	max	0	3	.003	2	.068	4	6.492e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	12	-1.306e-3	5	NC	1	1291.553	14
63	13	max	0	3	.004	2	.074	4	6.924e-4	1	NC	1	NC	1
64		min	0	2	-.008	3	0	12	-1.316e-3	5	NC	1	1176.832	14
65	14	max	.001	3	.005	2	.08	4	7.356e-4	1	NC	1	NC	1
66		min	0	2	-.008	3	0	12	-1.327e-3	5	9523.667	2	1081.009	14
67	15	max	.001	3	.006	2	.086	4	7.788e-4	1	NC	1	NC	2
68		min	0	2	-.008	3	0	12	-1.338e-3	5	8039.685	2	999.892	14
69	16	max	.001	3	.007	2	.092	4	8.22e-4	1	NC	3	NC	2
70		min	0	2	-.008	3	0	12	-1.348e-3	5	6883.618	2	930.415	14
71	17	max	.001	3	.008	2	.098	4	8.651e-4	1	NC	3	NC	2
72		min	-.001	2	-.008	3	0	12	-1.359e-3	5	5974.111	2	870.286	14
73	18	max	.001	3	.009	2	.104	4	9.083e-4	1	NC	3	NC	2
74		min	-.001	2	-.008	3	0	12	-1.369e-3	5	5252.472	2	817.756	14
75	19	max	.001	3	.01	2	.11	4	9.515e-4	1	NC	3	NC	2
76		min	-.001	2	-.008	3	0	12	-1.38e-3	5	4676.077	2	771.469	14
77	M4	1	max	.003	1	.01	2	12	6.574e-3	5	NC	1	NC	3
78		min	0	3	-.008	3	-.116	4	-1.252e-3	1	NC	1	167.039	4
79	2	max	.002	1	.01	2	0	12	6.574e-3	5	NC	1	NC	3
80		min	0	3	-.008	3	-.106	4	-1.252e-3	1	NC	1	182.099	4
81	3	max	.002	1	.009	2	0	12	6.574e-3	5	NC	1	NC	3
82		min	0	3	-.007	3	-.097	4	-1.252e-3	1	NC	1	200.025	4
83	4	max	.002	1	.008	2	0	12	6.574e-3	5	NC	1	NC	3
84		min	0	3	-.007	3	-.087	4	-1.252e-3	1	NC	1	221.572	4
85	5	max	.002	1	.008	2	0	12	6.574e-3	5	NC	1	NC	2
86		min	0	3	-.006	3	-.078	4	-1.252e-3	1	NC	1	247.769	4
87	6	max	.002	1	.007	2	0	12	6.574e-3	5	NC	1	NC	2
88		min	0	3	-.006	3	-.069	4	-1.252e-3	1	NC	1	280.044	4
89	7	max	.002	1	.007	2	0	12	6.574e-3	5	NC	1	NC	2
90		min	0	3	-.005	3	-.06	4	-1.252e-3	1	NC	1	320.436	4
91	8	max	.002	1	.006	2	0	12	6.574e-3	5	NC	1	NC	2
92		min	0	3	-.005	3	-.052	4	-1.252e-3	1	NC	1	371.926	4
93	9	max	.001	1	.006	2	0	12	6.574e-3	5	NC	1	NC	2
94		min	0	3	-.004	3	-.044	4	-1.252e-3	1	NC	1	439.027	4
95	10	max	.001	1	.005	2	0	12	6.574e-3	5	NC	1	NC	2
96		min	0	3	-.004	3	-.037	4	-1.252e-3	1	NC	1	528.83	4
97	11	max	.001	1	.004	2	0	12	6.574e-3	5	NC	1	NC	1
98		min	0	3	-.004	3	-.03	4	-1.252e-3	1	NC	1	653.015	4
99	12	max	0	1	.004	2	0	12	6.574e-3	5	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.013	4	1.745e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	2	-1.234e-3	4	NC	1	NC	1
159		4	max	0	3	.004	2	.019	4	1.938e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	2	-1.227e-3	4	NC	1	NC	1
161		5	max	.001	3	.006	2	.026	4	2.131e-5	1	NC	3	NC	1
162			min	-.001	2	-.007	3	0	2	-1.221e-3	4	8068.848	2	NC	1
163		6	max	.001	3	.007	2	.032	4	2.323e-5	1	NC	3	NC	1
164			min	-.001	2	-.009	3	0	2	-1.215e-3	4	6471.847	2	NC	1
165		7	max	.002	3	.009	2	.039	4	2.647e-5	3	NC	3	NC	1
166			min	-.002	2	-.01	3	0	2	-1.208e-3	4	5381.279	2	NC	1
167		8	max	.002	3	.01	2	.045	4	3.872e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-1.202e-3	4	4582.779	2	NC	1
169		9	max	.002	3	.012	2	.051	4	5.096e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	1	-1.196e-3	4	3969.521	2	NC	1
171		10	max	.002	3	.013	2	.058	4	6.321e-5	3	NC	3	NC	1
172			min	-.003	2	-.015	3	-.001	1	-1.189e-3	4	3482.301	2	NC	1
173		11	max	.003	3	.015	2	.064	4	7.545e-5	3	NC	3	NC	1
174			min	-.003	2	-.016	3	-.001	1	-1.183e-3	4	3085.594	2	NC	1
175		12	max	.003	3	.017	2	.07	4	8.77e-5	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-1.177e-3	4	2756.692	2	NC	1
177		13	max	.003	3	.019	2	.076	4	9.994e-5	3	NC	3	NC	1
178			min	-.004	2	-.018	3	-.002	1	-1.17e-3	4	2480.311	2	NC	1
179		14	max	.003	3	.021	2	.082	4	1.122e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	-.002	1	-1.164e-3	4	2245.703	2	NC	1
181		15	max	.004	3	.023	2	.087	4	1.244e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	-.002	1	-1.157e-3	4	2045.028	2	NC	1
183		16	max	.004	3	.025	2	.093	4	1.367e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	-.002	1	-1.151e-3	4	1872.39	2	NC	1
185		17	max	.004	3	.027	2	.099	4	1.489e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	-.002	1	-1.145e-3	4	1723.244	2	NC	1
187		18	max	.004	3	.029	2	.104	4	1.612e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	-.002	1	-1.138e-3	4	1594.014	2	NC	1
189		19	max	.005	3	.031	2	.11	4	1.734e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	-.003	1	-1.132e-3	4	1481.845	2	NC	1
191	M8	1	max	.007	1	.033	2	.003	1	6.357e-3	4	NC	1	NC	2
192			min	0	3	-.023	3	-.116	4	-2.08e-4	1	NC	1	167.017	4
193		2	max	.006	1	.031	2	.003	1	6.357e-3	4	NC	1	NC	2
194			min	0	3	-.022	3	-.106	4	-2.08e-4	1	NC	1	182.075	4
195		3	max	.006	1	.029	2	.002	1	6.357e-3	4	NC	1	NC	2
196			min	0	3	-.021	3	-.097	4	-2.08e-4	1	NC	1	199.998	4
197		4	max	.006	1	.027	2	.002	1	6.357e-3	4	NC	1	NC	2
198			min	0	3	-.02	3	-.087	4	-2.08e-4	1	NC	1	221.541	4
199		5	max	.005	1	.026	2	.002	1	6.357e-3	4	NC	1	NC	2
200			min	0	3	-.018	3	-.078	4	-2.08e-4	1	NC	1	247.733	4
201		6	max	.005	1	.024	2	.002	1	6.357e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.069	4	-2.08e-4	1	NC	1	280.003	4
203		7	max	.005	1	.022	2	.002	1	6.357e-3	4	NC	1	NC	1
204			min	0	3	-.016	3	-.06	4	-2.08e-4	1	NC	1	320.388	4
205		8	max	.004	1	.02	2	.001	1	6.357e-3	4	NC	1	NC	1
206			min	0	3	-.014	3	-.052	4	-2.08e-4	1	NC	1	371.869	4
207		9	max	.004	1	.018	2	.001	1	6.357e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.044	4	-2.08e-4	1	NC	1	438.958	4
209		10	max	.003	1	.016	2	0	1	6.357e-3	4	NC	1	NC	1
210			min	0	3	-.012	3	-.037	4	-2.08e-4	1	NC	1	528.746	4
211		11	max	.003	1	.015	2	0	1	6.357e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.03	4	-2.08e-4	1	NC	1	652.909	4
213		12	max	.003	1	.013	2	0	1	6.357e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.009	3	-.023	4	-2.08e-4	1	NC	1	831.82	4
215		13	max	.002	1	.011	2	0	1	6.357e-3	4	NC	1	NC	1
216			min	0	3	-.008	3	-.018	4	-2.08e-4	1	NC	1	1103.689	4
217		14	max	.002	1	.009	2	0	1	6.357e-3	4	NC	1	NC	1
218			min	0	3	-.007	3	-.012	4	-2.08e-4	1	NC	1	1547.336	4
219		15	max	.002	1	.007	2	0	1	6.357e-3	4	NC	1	NC	1
220			min	0	3	-.005	3	-.008	4	-2.08e-4	1	NC	1	2347.939	4
221		16	max	.001	1	.005	2	0	1	6.357e-3	4	NC	1	NC	1
222			min	0	3	-.004	3	-.005	4	-2.08e-4	1	NC	1	4031.925	4
223		17	max	0	1	.004	2	0	1	6.357e-3	4	NC	1	NC	1
224			min	0	3	-.003	3	-.002	4	-2.08e-4	1	NC	1	8643.85	4
225		18	max	0	1	.002	2	0	1	6.357e-3	4	NC	1	NC	1
226			min	0	3	-.001	3	0	4	-2.08e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	6.357e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.08e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.009	2	0	3	1.27e-3	1	NC	3	NC	1
230			min	-.003	3	-.008	3	-.009	4	-1.978e-4	3	4168.396	2	NC	1
231		2	max	.003	1	.008	2	0	3	1.203e-3	1	NC	3	NC	1
232			min	-.003	3	-.008	3	-.009	4	-1.922e-4	3	4517.821	2	NC	1
233		3	max	.003	1	.007	2	0	3	1.136e-3	1	NC	3	NC	1
234			min	-.003	3	-.007	3	-.009	4	-1.866e-4	3	4927.834	2	NC	1
235		4	max	.003	1	.007	2	0	3	1.069e-3	1	NC	3	NC	1
236			min	-.003	3	-.007	3	-.009	4	-1.811e-4	3	5411.877	2	NC	1
237		5	max	.003	1	.006	2	0	3	1.051e-3	14	NC	3	NC	1
238			min	-.002	3	-.007	3	-.009	4	-1.755e-4	3	5987.549	2	NC	1
239		6	max	.002	1	.005	2	0	3	1.112e-3	4	NC	1	NC	1
240			min	-.002	3	-.006	3	-.009	4	-1.699e-4	3	6678.24	2	NC	1
241		7	max	.002	1	.005	2	0	3	1.184e-3	4	NC	1	NC	1
242			min	-.002	3	-.006	3	-.009	4	-1.644e-4	3	7515.611	2	NC	1
243		8	max	.002	1	.004	2	0	3	1.256e-3	4	NC	1	NC	1
244			min	-.002	3	-.006	3	-.008	4	-1.588e-4	3	8543.428	2	NC	1
245		9	max	.002	1	.004	2	0	3	1.328e-3	4	NC	1	NC	1
246			min	-.002	3	-.005	3	-.008	4	-1.532e-4	3	9823.72	2	NC	1
247		10	max	.002	1	.003	2	0	3	1.4e-3	4	NC	1	NC	1
248			min	-.002	3	-.005	3	-.008	4	-1.477e-4	3	NC	1	NC	1
249		11	max	.002	1	.003	2	0	3	1.471e-3	4	NC	1	NC	1
250			min	-.001	3	-.004	3	-.007	4	-1.421e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	1.543e-3	4	NC	1	NC	1
252			min	-.001	3	-.004	3	-.007	4	-1.365e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	1.615e-3	4	NC	1	NC	1
254			min	-.001	3	-.004	3	-.006	4	-1.31e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.687e-3	4	NC	1	NC	1
256			min	0	3	-.003	3	-.005	4	-1.254e-4	3	NC	1	NC	1
257		15	max	0	1	.001	2	0	3	1.759e-3	4	NC	1	NC	1
258			min	0	3	-.002	3	-.004	4	-1.199e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.831e-3	4	NC	1	NC	1
260			min	0	3	-.002	3	-.003	4	-1.143e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.903e-3	4	NC	1	NC	1
262			min	0	3	-.001	3	-.002	4	-1.087e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.975e-3	4	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-1.032e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.047e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-9.759e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	4.539e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-9.539e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.005	4	3.04e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.071e-3	4	NC	1	9331.85	4



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.01	4	1.54e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-1.189e-3	4	NC	1	4638.634	4
273	4	max	0	3	0	2	.015	4	4.013e-7	3	NC	1	NC	1
274		min	0	2	-.003	3	0	1	-1.306e-3	4	NC	1	3077.837	4
275	5	max	0	3	0	2	.02	4	-9.944e-6	12	NC	1	NC	1
276		min	0	2	-.003	3	-.001	1	-1.423e-3	4	NC	1	2299.738	4
277	6	max	0	3	0	2	.025	5	-1.95e-5	12	NC	1	NC	1
278		min	0	2	-.004	3	-.002	1	-1.541e-3	4	NC	1	1832.636	5
279	7	max	0	3	.001	2	.03	5	-2.907e-5	12	NC	1	NC	1
280		min	0	2	-.005	3	-.003	1	-1.658e-3	4	NC	1	1517.751	5
281	8	max	0	3	.001	2	.036	5	-3.863e-5	12	NC	1	NC	1
282		min	0	2	-.006	3	-.004	1	-1.775e-3	4	NC	1	1293.967	5
283	9	max	0	3	.002	2	.041	5	-4.819e-5	12	NC	1	NC	2
284		min	0	2	-.006	3	-.005	1	-1.893e-3	4	NC	1	1126.931	5
285	10	max	0	3	.002	2	.046	5	-5.775e-5	12	NC	1	NC	2
286		min	0	2	-.007	3	-.007	1	-2.01e-3	4	NC	1	997.567	5
287	11	max	0	3	.003	2	.051	5	-6.731e-5	12	NC	1	NC	2
288		min	0	2	-.007	3	-.008	1	-2.128e-3	4	NC	1	894.437	5
289	12	max	0	3	.003	2	.057	5	-7.597e-5	10	NC	1	NC	2
290		min	0	2	-.007	3	-.01	1	-2.245e-3	4	NC	1	810.264	5
291	13	max	0	3	.004	2	.062	5	-8.225e-5	10	NC	1	NC	2
292		min	0	2	-.008	3	-.011	1	-2.362e-3	4	NC	1	740.203	5
293	14	max	.001	3	.005	2	.068	5	-8.854e-5	10	NC	1	NC	2
294		min	0	2	-.008	3	-.013	1	-2.48e-3	4	9320.872	2	680.901	5
295	15	max	.001	3	.006	2	.073	5	-9.482e-5	10	NC	1	NC	3
296		min	0	2	-.008	3	-.014	1	-2.597e-3	4	7921.913	2	629.965	5
297	16	max	.001	3	.007	2	.079	5	-1.011e-4	10	NC	3	NC	3
298		min	0	2	-.008	3	-.016	1	-2.714e-3	4	6819.077	2	585.645	5
299	17	max	.001	3	.008	2	.084	5	-1.074e-4	10	NC	3	NC	3
300		min	-.001	2	-.008	3	-.017	1	-2.832e-3	4	5943.161	2	546.628	5
301	18	max	.001	3	.009	2	.09	5	-1.137e-4	10	NC	3	NC	3
302		min	-.001	2	-.008	3	-.018	1	-2.949e-3	4	5242.861	2	511.915	5
303	19	max	.001	3	.01	2	.096	5	-1.2e-4	10	NC	3	NC	3
304		min	-.001	2	-.008	3	-.02	1	-3.066e-3	4	4680.09	2	480.731	5
305	M12	1	max	.003	1	.01	.016	1	8.069e-3	4	NC	1	NC	3
306		min	0	3	-.008	3	-.106	5	1.117e-4	10	NC	1	182.832	5
307	2	max	.002	1	.01	2	.015	1	8.069e-3	4	NC	1	NC	3
308		min	0	3	-.008	3	-.097	5	1.117e-4	10	NC	1	199.312	5
309	3	max	.002	1	.009	2	.013	1	8.069e-3	4	NC	1	NC	3
310		min	0	3	-.007	3	-.088	5	1.117e-4	10	NC	1	218.929	5
311	4	max	.002	1	.008	2	.012	1	8.069e-3	4	NC	1	NC	3
312		min	0	3	-.007	3	-.08	5	1.117e-4	10	NC	1	242.507	5
313	5	max	.002	1	.008	2	.011	1	8.069e-3	4	NC	1	NC	3
314		min	0	3	-.006	3	-.071	5	1.117e-4	10	NC	1	271.172	5
315	6	max	.002	1	.007	2	.01	1	8.069e-3	4	NC	1	NC	3
316		min	0	3	-.006	3	-.063	5	1.117e-4	10	NC	1	306.489	5
317	7	max	.002	1	.007	2	.008	1	8.069e-3	4	NC	1	NC	3
318		min	0	3	-.005	3	-.055	5	1.117e-4	10	NC	1	350.687	5
319	8	max	.002	1	.006	2	.007	1	8.069e-3	4	NC	1	NC	3
320		min	0	3	-.005	3	-.047	5	1.117e-4	10	NC	1	407.029	5
321	9	max	.001	1	.006	2	.006	1	8.069e-3	4	NC	1	NC	3
322		min	0	3	-.004	3	-.04	5	1.117e-4	10	NC	1	480.451	5
323	10	max	.001	1	.005	2	.005	1	8.069e-3	4	NC	1	NC	2
324		min	0	3	-.004	3	-.033	5	1.117e-4	10	NC	1	578.712	5
325	11	max	.001	1	.004	2	.004	1	8.069e-3	4	NC	1	NC	2
326		min	0	3	-.004	3	-.027	5	1.117e-4	10	NC	1	714.593	5
327	12	max	0	1	.004	2	.003	1	8.069e-3	4	NC	1	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.021	5	1.117e-4	10	NC	1	910.385	5
329		max	0	1	.003	2	.002	1	8.069e-3	4	NC	1	NC	2
330		min	0	3	-.003	3	-.016	5	1.117e-4	10	NC	1	1207.904	5
331		max	0	1	.003	2	.002	1	8.069e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.011	5	1.117e-4	10	NC	1	1693.4	5
333		max	0	1	.002	2	.001	1	8.069e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.008	5	1.117e-4	10	NC	1	2569.512	5
335		max	0	1	.002	2	0	1	8.069e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.004	5	1.117e-4	10	NC	1	4412.3	5
337		max	0	1	.001	2	0	1	8.069e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.002	5	1.117e-4	10	NC	1	9459.063	5
339		max	0	1	0	2	0	1	8.069e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	1.117e-4	10	NC	1	NC	1
341		max	0	1	0	1	0	1	8.069e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.117e-4	10	NC	1	NC	1
343	M1	max	.008	3	.024	3	.011	5	1.813e-2	1	NC	1	NC	1
344		min	-.008	2	-.028	1	-.006	1	-1.699e-2	3	NC	1	NC	1
345		max	.007	3	.014	3	.016	5	8.533e-3	1	NC	4	NC	2
346		min	-.008	2	-.016	1	-.014	1	-8.418e-3	3	3665.668	1	6056.259	1
347		max	.007	3	.004	3	.021	5	6.832e-4	5	NC	4	NC	3
348		min	-.008	2	-.004	1	-.019	1	-8.803e-4	1	1895.719	1	3671.352	1
349		max	.007	3	.006	1	.026	5	6.944e-4	5	NC	5	NC	3
350		min	-.008	2	-.004	3	-.021	1	-7.43e-4	1	1341.004	1	3036.737	1
351		max	.007	3	.015	1	.032	5	7.057e-4	5	NC	5	NC	3
352		min	-.008	2	-.011	3	-.022	1	-6.058e-4	1	1074.417	1	2272.709	5
353		max	.007	3	.022	1	.038	5	7.169e-4	5	NC	5	NC	3
354		min	-.008	2	-.016	3	-.021	1	-4.685e-4	1	923.777	1	1745.622	5
355		max	.007	3	.027	1	.045	5	7.282e-4	5	NC	5	NC	3
356		min	-.008	2	-.02	3	-.018	1	-3.312e-4	1	832.617	1	1404.364	5
357		max	.007	3	.031	1	.051	5	7.394e-4	5	NC	5	NC	2
358		min	-.008	2	-.023	3	-.015	1	-1.939e-4	1	777.458	1	1167.855	5
359		max	.007	3	.033	1	.058	5	7.507e-4	5	NC	5	NC	2
360		min	-.008	2	-.024	3	-.01	1	-5.667e-5	1	747.34	1	991.825	4
361		max	.007	3	.034	1	.065	5	7.708e-4	4	NC	5	NC	1
362		min	-.008	2	-.025	3	-.006	1	9.239e-6	10	737.211	1	846.144	4
363		max	.007	3	.033	1	.073	4	8.129e-4	4	NC	5	NC	1
364		min	-.008	2	-.024	3	-.001	1	2.222e-5	10	745.54	1	737.188	4
365		max	.007	3	.031	1	.081	4	8.551e-4	4	NC	5	NC	2
366		min	-.008	2	-.022	3	0	10	3.058e-5	12	773.684	1	653.819	4
367		max	.007	3	.027	1	.089	4	8.972e-4	4	NC	5	NC	2
368		min	-.008	2	-.019	3	0	12	3.464e-5	12	826.46	1	588.941	4
369		max	.007	3	.021	1	.096	4	9.393e-4	4	NC	5	NC	3
370		min	-.008	2	-.015	3	0	12	3.87e-5	12	914.43	1	537.863	4
371		max	.007	3	.014	1	.103	4	9.814e-4	4	NC	5	NC	3
372		min	-.008	2	-.01	3	0	12	4.276e-5	12	1060.245	1	497.393	4
373		max	.007	3	.005	1	.11	4	1.392e-3	4	NC	5	NC	3
374		min	-.008	2	-.004	3	0	12	4.556e-5	12	1317.944	1	465.315	4
375		max	.007	3	.003	3	.116	4	1.058e-2	4	NC	4	NC	3
376		min	-.008	2	-.006	2	0	12	-3.184e-6	2	1850.158	1	440.097	4
377		max	.007	3	.011	3	.121	4	1.018e-2	1	NC	4	NC	2
378		min	-.008	2	-.018	2	0	10	-3.722e-3	3	3566.193	1	420.604	4
379		max	.007	3	.019	3	.124	4	2.055e-2	1	NC	1	NC	1
380		min	-.008	2	-.031	2	-.004	1	-7.543e-3	3	NC	1	406.544	4
381	M5	max	.022	3	.073	3	.011	5	5.973e-6	4	NC	1	NC	1
382		min	-.027	2	-.087	1	-.007	1	5.779e-8	10	NC	1	NC	1
383		max	.022	3	.042	3	.015	5	3.435e-4	5	NC	5	NC	1
384		min	-.028	2	-.049	1	-.006	1	-8.051e-5	1	1215.194	1	NC	1



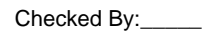
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.022	3	.013	3	.021	5	6.754e-4	5	NC	5	NC	1
386		min	-.028	2	-.013	1	-.005	1	-1.602e-4	1	625.358	1	NC	1
387	4	max	.022	3	.017	1	.027	5	7.012e-4	5	NC	5	NC	1
388		min	-.028	2	-.011	3	-.004	1	-1.501e-4	1	440.959	1	NC	1
389	5	max	.022	3	.044	1	.033	5	7.271e-4	5	NC	15	NC	1
390		min	-.028	2	-.03	3	-.004	1	-1.401e-4	1	352.25	1	NC	1
391	6	max	.022	3	.065	1	.04	5	7.529e-4	5	NC	15	NC	1
392		min	-.028	2	-.046	3	-.003	1	-1.3e-4	1	302.007	1	NC	1
393	7	max	.021	3	.082	1	.047	5	7.788e-4	5	NC	15	NC	1
394		min	-.028	2	-.058	3	-.003	1	-1.2e-4	1	271.466	1	NC	1
395	8	max	.021	3	.094	1	.054	5	8.046e-4	5	NC	15	NC	1
396		min	-.028	2	-.066	3	-.002	1	-1.099e-4	1	252.823	1	NC	1
397	9	max	.021	3	.102	1	.062	5	8.305e-4	5	9977.466	15	NC	1
398		min	-.028	2	-.07	3	-.002	1	-9.986e-5	1	242.426	1	NC	1
399	10	max	.021	3	.104	1	.069	5	8.563e-4	5	9872.337	15	NC	1
400		min	-.028	2	-.071	3	-.002	1	-8.98e-5	1	238.579	1	NC	1
401	11	max	.021	3	.102	1	.076	5	8.821e-4	5	NC	15	NC	1
402		min	-.028	2	-.069	3	-.002	1	-7.974e-5	1	240.748	1	NC	1
403	12	max	.021	3	.095	1	.084	5	9.08e-4	5	NC	15	NC	1
404		min	-.028	2	-.063	3	-.002	1	-6.969e-5	1	249.342	1	NC	1
405	13	max	.021	3	.083	1	.091	5	9.338e-4	5	NC	15	NC	1
406		min	-.028	2	-.054	3	-.002	1	-5.963e-5	1	265.901	1	NC	1
407	14	max	.021	3	.065	1	.097	4	9.597e-4	5	NC	15	NC	1
408		min	-.028	2	-.042	3	-.002	1	-4.957e-5	1	293.833	1	9221.296	4
409	15	max	.021	3	.043	1	.104	4	9.855e-4	5	NC	15	NC	1
410		min	-.028	2	-.028	3	-.002	1	-4.291e-5	2	340.499	1	9081.936	4
411	16	max	.021	3	.015	1	.11	4	1.378e-3	5	NC	5	NC	1
412		min	-.028	2	-.01	3	-.003	1	-4.133e-5	2	423.641	1	9803.063	4
413	17	max	.021	3	.01	3	.116	4	1.057e-2	4	NC	5	NC	1
414		min	-.028	2	-.018	2	-.003	1	-2.853e-4	1	598.126	1	NC	1
415	18	max	.021	3	.032	3	.121	4	5.42e-3	4	NC	5	NC	1
416		min	-.028	2	-.056	1	-.003	1	-1.463e-4	1	1159.759	1	NC	1
417	19	max	.021	3	.055	3	.125	4	1.807e-6	5	NC	1	NC	1
418		min	-.028	2	-.097	1	-.003	1	-1.506e-7	3	NC	1	NC	1
419	M9	1	max	.008	.024	3	.009	5	1.699e-2	3	NC	1	NC	1
420		min	-.008	2	-.028	1	-.008	1	-1.812e-2	1	NC	1	NC	1
421	2	max	.008	3	.014	3	.008	5	8.414e-3	3	NC	4	NC	2
422		min	-.008	2	-.016	1	-.002	1	-8.859e-3	1	3666.435	1	7217.144	1
423	3	max	.008	3	.004	3	.009	4	2.326e-4	1	NC	4	NC	2
424		min	-.008	2	-.004	1	0	3	-3.407e-6	3	1896.122	1	4504.634	1
425	4	max	.008	3	.006	1	.011	4	1.153e-4	1	NC	5	NC	3
426		min	-.008	2	-.004	3	0	3	-1.287e-5	3	1341.281	1	3837.834	1
427	5	max	.007	3	.015	1	.015	4	5.761e-5	5	NC	5	NC	3
428		min	-.008	2	-.011	3	0	3	-2.234e-5	3	1074.62	1	3832.665	1
429	6	max	.007	3	.022	1	.019	4	5.998e-5	5	NC	5	NC	3
430		min	-.008	2	-.016	3	-.001	3	-1.192e-4	1	923.933	1	3574.514	14
431	7	max	.007	3	.027	1	.023	4	6.234e-5	5	NC	5	NC	2
432		min	-.008	2	-.02	3	-.002	3	-2.365e-4	1	832.739	1	2715.18	4
433	8	max	.007	3	.031	1	.029	4	6.471e-5	5	NC	5	NC	1
434		min	-.008	2	-.023	3	-.002	3	-3.537e-4	1	777.553	1	2059.237	4
435	9	max	.007	3	.033	1	.035	5	6.708e-5	5	NC	5	NC	1
436		min	-.008	2	-.025	3	-.005	1	-4.71e-4	1	747.414	1	1620.361	4
437	10	max	.007	3	.034	1	.043	5	6.945e-5	5	NC	5	NC	1
438		min	-.008	2	-.025	3	-.009	1	-5.883e-4	1	737.266	1	1312.33	4
439	11	max	.007	3	.033	1	.051	5	7.181e-5	5	NC	5	NC	2
440		min	-.008	2	-.024	3	-.013	1	-7.055e-4	1	745.578	1	1087.758	4
441	12	max	.007	3	.031	1	.059	5	7.418e-5	5	NC	5	NC	2





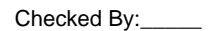
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.003	1	.17	3	.168	1	7.363e-3	1	NC	5	NC	3
500		min	-.125	4	-.439	1	.011	10	-4.319e-3	3	499.128	1	1174.325	1
501	4	max	.003	1	.213	3	.253	1	8.518e-3	1	NC	5	NC	12
502		min	-.125	4	-.555	1	.018	10	-4.938e-3	3	389.282	1	790.557	1
503	5	max	.003	1	.227	3	.294	1	9.674e-3	1	NC	15	NC	12
504		min	-.125	4	-.589	1	.021	10	-5.557e-3	3	365.734	1	680.901	1
505	6	max	.003	1	.211	3	.281	1	1.083e-2	1	NC	5	NC	12
506		min	-.125	4	-.542	1	.018	10	-6.176e-3	3	398.85	1	711.911	1
507	7	max	.003	1	.173	3	.217	1	1.199e-2	1	NC	5	NC	5
508		min	-.125	4	-.432	1	.01	15	-6.795e-3	3	508.472	1	917.919	1
509	8	max	.003	1	.123	3	.12	1	1.314e-2	1	NC	5	NC	5
510		min	-.125	4	-.288	1	0	10	-7.414e-3	3	791.787	1	1629.385	1
511	9	max	.003	1	.076	3	.024	3	1.43e-2	1	NC	5	NC	2
512		min	-.125	4	-.157	1	-.011	10	-8.033e-3	3	1618.617	1	7000.031	1
513	10	max	.003	1	.055	3	.021	3	1.545e-2	1	NC	4	NC	1
514		min	-.125	4	-.097	1	-.028	2	-8.652e-3	3	3083.067	1	NC	1
515	11	max	.004	1	.076	3	.023	14	1.43e-2	1	NC	5	NC	2
516		min	-.125	4	-.157	1	-.011	10	-8.033e-3	3	1618.617	1	7556.017	1
517	12	max	.004	1	.123	3	.116	1	1.314e-2	1	NC	5	NC	3
518		min	-.125	4	-.288	1	0	10	-7.413e-3	3	791.787	1	1683.503	1
519	13	max	.004	1	.173	3	.211	1	1.199e-2	1	NC	5	NC	5
520		min	-.125	4	-.432	1	.01	10	-6.794e-3	3	508.473	1	941.319	1
521	14	max	.004	1	.211	3	.275	1	1.083e-2	1	NC	5	NC	5
522		min	-.125	4	-.542	1	.008	15	-6.174e-3	3	398.851	1	728.505	1
523	15	max	.004	1	.227	3	.287	1	9.676e-3	1	NC	15	NC	5
524		min	-.125	4	-.589	1	.001	15	-5.555e-3	3	365.734	1	696.989	1
525	16	max	.004	1	.213	3	.246	1	8.521e-3	1	NC	5	NC	3
526		min	-.124	4	-.555	1	-.007	5	-4.935e-3	3	389.282	1	811.223	1
527	17	max	.004	1	.17	3	.163	1	7.366e-3	1	NC	5	NC	3
528		min	-.124	4	-.439	1	-.014	5	-4.315e-3	3	499.128	1	1211.894	1
529	18	max	.004	1	.102	3	.065	1	6.211e-3	1	NC	5	NC	3
530		min	-.124	4	-.256	1	-.013	5	-3.696e-3	3	907.345	1	2897.667	1
531	19	max	.004	1	.019	3	.007	3	5.056e-3	1	NC	1	NC	1
532		min	-.124	4	-.031	2	-.008	2	-3.076e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.441e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.02e-4	5	NC	1	NC	1
535	2	max	0	1	-.002	15	.015	4	8.46e-4	3	NC	5	NC	1
536		min	-.001	5	-.024	1	0	3	-7.2e-4	1	4552.337	6	7285.892	4
537	3	max	0	1	-.004	15	.032	4	1.348e-3	3	NC	5	NC	1
538		min	-.002	5	-.047	1	-.003	3	-1.389e-3	1	2316.529	6	3321.698	4
539	4	max	0	1	-.006	15	.051	4	1.85e-3	3	NC	15	NC	9
540		min	-.003	5	-.069	1	-.007	3	-2.057e-3	1	1589.274	6	2119.653	4
541	5	max	0	1	-.008	15	.068	4	2.351e-3	3	NC	15	NC	9
542		min	-.004	5	-.088	1	-.011	3	-2.726e-3	1	1240.127	6	1580.027	4
543	6	max	0	1	-.01	15	.083	4	2.853e-3	3	9966.167	15	9221.725	9
544		min	-.005	5	-.105	1	-.016	3	-3.395e-3	1	1043.698	6	1297.017	4
545	7	max	0	1	-.011	15	.094	4	3.355e-3	3	8838.192	15	7235.794	9
546		min	-.007	5	-.118	1	-.021	3	-4.063e-3	1	925.572	6	1141.886	4
547	8	max	0	1	-.012	15	.101	4	3.857e-3	3	8161.238	15	5983.038	9
548		min	-.008	5	-.128	1	-.026	3	-4.732e-3	1	854.678	6	1063.939	4
549	9	max	0	1	-.012	15	.103	4	4.359e-3	3	7796.861	15	5161.435	9
550		min	-.009	5	-.134	1	-.03	3	-5.401e-3	1	816.519	6	1042.378	4
551	10	max	0	1	-.012	15	.1	4	4.861e-3	3	7681.589	15	4618.702	9
552		min	-.01	5	-.136	1	-.033	3	-6.069e-3	1	804.447	6	1071.775	4
553	11	max	0	1	-.012	15	.093	4	5.362e-3	3	7796.861	15	4274.755	9
554		min	-.011	5	-.135	1	-.036	3	-6.738e-3	1	816.519	6	1158.71	4
555	12	max	0	1	-.011	15	.081	4	5.864e-3	3	8161.238	15	4089.995	9





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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

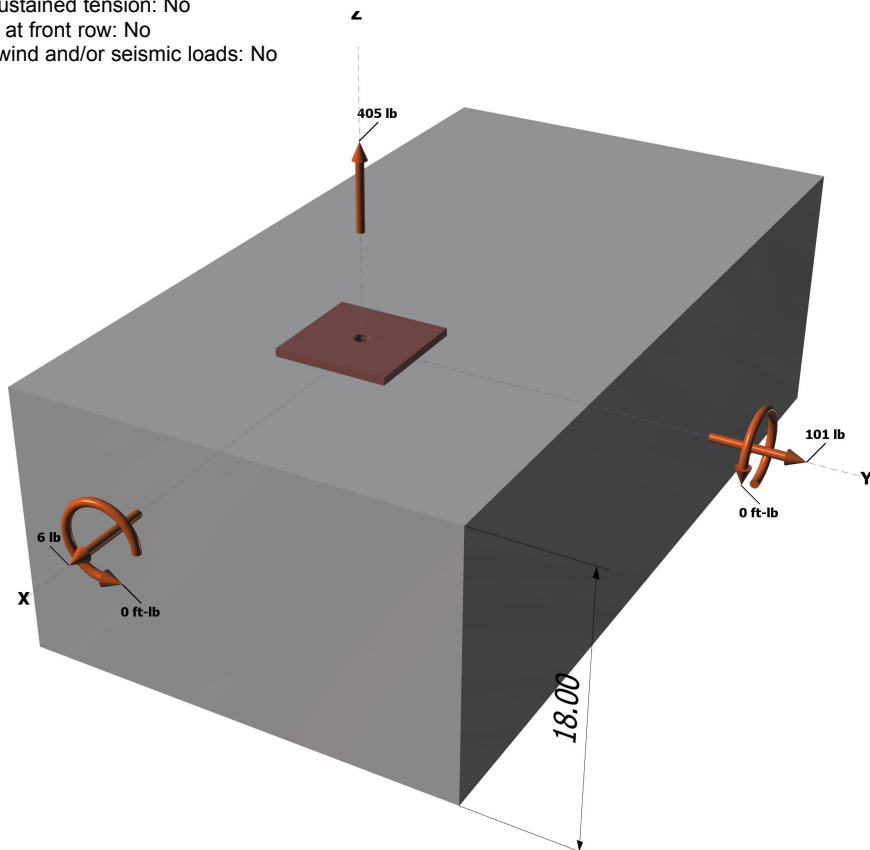
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



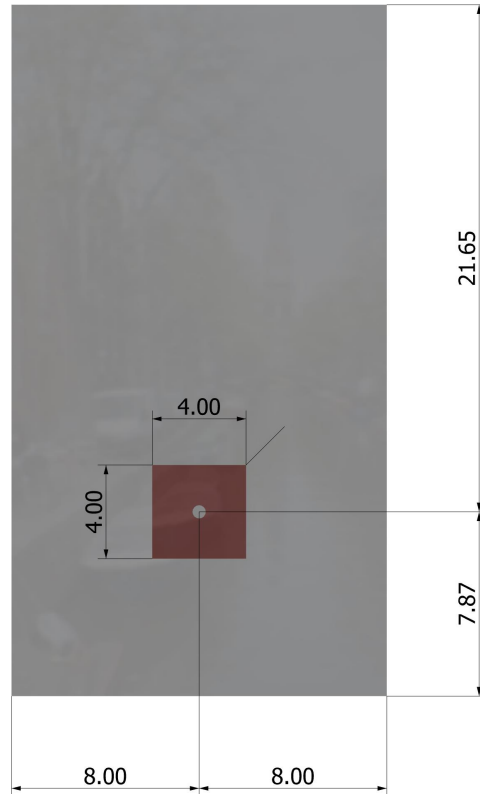
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

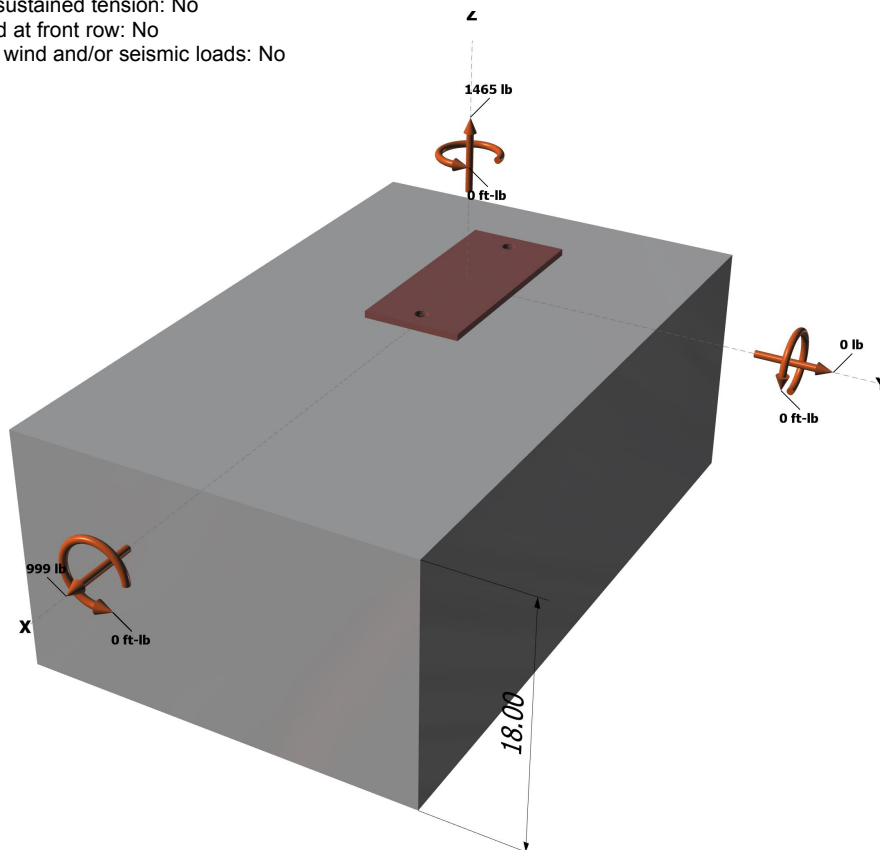
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



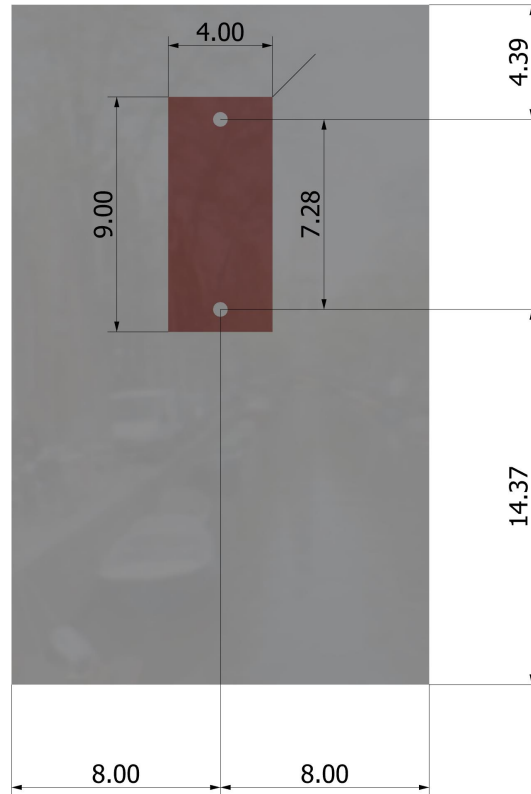
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.