



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	140 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 30.77 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.712 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.766 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.640 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	14%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.113 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 37 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

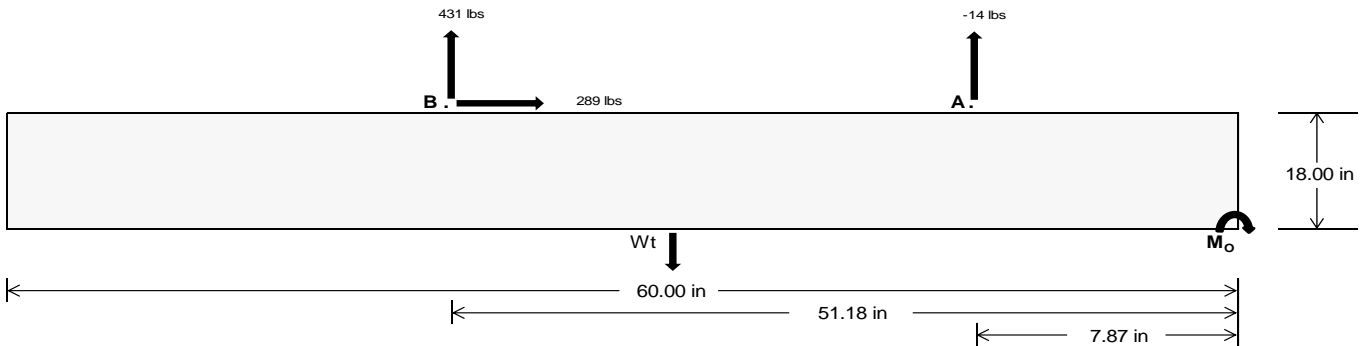
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>22.05</u>	<u>1868.84</u>	k
Compressive Load =	<u>926.13</u>	<u>1246.42</u>	k
Lateral Load =	<u>1.78</u>	<u>1254.29</u>	k
Moment (Weak Axis) =	<u>0.00</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27132.9$ in-lbs
Resisting Force Required = 904.43 lbs
S.F. = 1.67
Weight Required = 1507.38 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 289.32 lbs
Friction = 0.4
Weight Required = 723.29 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 289.32 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

	Ballast Width			
	21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	326 lbs	326 lbs	326 lbs	326 lbs	327 lbs	327 lbs	327 lbs	327 lbs	457 lbs	457 lbs	457 lbs	457 lbs	28 lbs	28 lbs	28 lbs	28 lbs
F_B	213 lbs	213 lbs	213 lbs	213 lbs	537 lbs	537 lbs	537 lbs	537 lbs	540 lbs	540 lbs	540 lbs	540 lbs	-861 lbs	-861 lbs	-861 lbs	-861 lbs
F_V	35 lbs	35 lbs	35 lbs	35 lbs	523 lbs	523 lbs	523 lbs	523 lbs	415 lbs	415 lbs	415 lbs	415 lbs	-579 lbs	-579 lbs	-579 lbs	-579 lbs
P_{total}	2442 lbs	2533 lbs	2623 lbs	2714 lbs	2768 lbs	2859 lbs	2949 lbs	3040 lbs	2900 lbs	2991 lbs	3081 lbs	3172 lbs	309 lbs	363 lbs	417 lbs	472 lbs
M	279 lbs-ft	279 lbs-ft	279 lbs-ft	279 lbs-ft	439 lbs-ft	439 lbs-ft	439 lbs-ft	439 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	512 lbs-ft	703 lbs-ft	703 lbs-ft	703 lbs-ft	703 lbs-ft
e	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.28 ft	1.94 ft	1.68 ft	1.49 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	240.8 psf	239.8 psf	238.8 psf	237.9 psf	256.1 psf	254.4 psf	252.7 psf	251.3 psf	261.2 psf	259.2 psf	257.4 psf	255.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	317.4 psf	312.8 psf	308.7 psf	304.9 psf	376.6 psf	369.3 psf	362.7 psf	356.7 psf	401.7 psf	393.3 psf	385.6 psf	378.6 psf	530.1 psf	234.4 psf	178.0 psf	155.8 psf

Maximum Bearing Pressure = 530 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

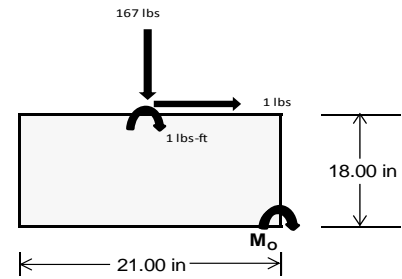
Overturning Check

$M_o = 144.2 \text{ ft-lbs}$
 Resisting Force Required = 164.81 lbs
 S.F. = 1.67
 Weight Required = 274.68 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	58 lbs	139 lbs	55 lbs	167 lbs	452 lbs	164 lbs	17 lbs	41 lbs	16 lbs
F_v	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2414 lbs	2495 lbs	2411 lbs	2410 lbs	2695 lbs	2407 lbs	706 lbs	730 lbs	705 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	275.8 sqft	285.1 sqft	275.5 sqft	274.6 sqft	307.8 sqft	274.9 sqft	80.6 sqft	83.4 sqft	80.6 sqft
f_{max}	276.1 psf	285.2 psf	275.6 psf	276.3 psf	308.3 psf	275.3 psf	80.7 psf	83.4 psf	80.6 psf



Maximum Bearing Pressure = 308 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.694 k
Allowable Uplift =	1.214 k
Utilization =	<u>57%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.118 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.712 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

Diagonal Strut

Maximum Axial Load =	0.766 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



Rear Strut

Maximum Axial Load =	1.138 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.113 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.01 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &22.2924 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.453 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	268.073	2	299.305	2	0	10	0	15	0	1	0	1
2		min	-314.832	3	-450.581	3	-.135	3	0	3	0	1	0	1
3	N7	max	.025	3	269.686	1	-.03	10	0	15	0	1	0	1
4		min	-.139	2	10.965	15	-.664	1	-.001	1	0	1	0	1
5	N15	max	.16	3	712.406	1	.229	9	0	1	0	1	0	1
6		min	-1.369	2	24.092	15	-.724	3	-.001	3	0	1	0	1
7	N16	max	883.149	2	958.785	2	0	2	0	9	0	1	0	1
8		min	-964.835	3	-1437.566	3	-87.573	3	0	3	0	1	0	1
9	N23	max	.026	3	269.782	1	1.138	1	.002	1	0	1	0	1
10		min	-.139	2	11.083	15	.03	10	0	10	0	1	0	1
11	N24	max	268.074	2	301.919	2	88.329	3	0	1	0	1	0	1
12		min	-315.402	3	-449.61	3	0	10	0	3	0	1	0	1
13	Totals:	max	1417.65	2	2631.596	2	0	9						
14		min	-1594.858	3	-2157.577	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	201.668	2	.678	4	.151	1	0	10	0	15	0	1
2			min	-371.708	3	.16	15	-.056	3	0	1	0	1	0	1
3		2	max	201.803	2	.621	4	.151	1	0	10	0	15	0	15
4			min	-371.606	3	.146	15	-.056	3	0	1	0	1	0	4
5		3	max	201.938	2	.563	4	.151	1	0	10	0	15	0	15
6			min	-371.505	3	.133	15	-.056	3	0	1	0	3	0	4
7		4	max	202.073	2	.506	4	.151	1	0	10	0	15	0	15
8			min	-371.404	3	.119	15	-.056	3	0	1	0	3	0	4
9		5	max	202.208	2	.448	4	.151	1	0	10	0	9	0	15
10			min	-371.303	3	.105	15	-.056	3	0	1	0	3	0	4
11		6	max	202.342	2	.391	4	.151	1	0	10	0	9	0	15
12			min	-371.202	3	.092	15	-.056	3	0	1	0	3	0	4
13		7	max	202.477	2	.333	4	.151	1	0	10	0	1	0	15
14			min	-371.101	3	.078	15	-.056	3	0	1	0	3	0	4
15		8	max	202.612	2	.276	4	.151	1	0	10	0	1	0	15
16			min	-371	3	.065	15	-.056	3	0	1	0	3	0	4
17		9	max	202.747	2	.218	4	.151	1	0	10	0	1	0	15
18			min	-370.898	3	.051	15	-.056	3	0	1	0	3	0	4
19		10	max	202.882	2	.161	4	.151	1	0	10	0	1	0	15
20			min	-370.797	3	.038	15	-.056	3	0	1	0	3	0	4
21		11	max	203.017	2	.11	2	.151	1	0	10	0	1	0	15
22			min	-370.696	3	.016	12	-.056	3	0	1	0	3	0	4
23		12	max	203.152	2	.066	2	.151	1	0	10	0	1	0	15
24			min	-370.595	3	-.014	3	-.056	3	0	1	0	3	0	4
25		13	max	203.287	2	.021	2	.151	1	0	10	0	1	0	15
26			min	-370.494	3	-.048	3	-.056	3	0	1	0	3	0	4
27		14	max	203.421	2	-.016	15	.151	1	0	10	0	1	0	15
28			min	-370.393	3	-.081	3	-.056	3	0	1	0	3	0	4
29		15	max	203.556	2	-.03	15	.151	1	0	10	0	1	0	15
30			min	-370.292	3	-.126	4	-.056	3	0	1	0	3	0	4
31		16	max	203.691	2	-.043	15	.151	1	0	10	0	1	0	15
32			min	-370.19	3	-.184	4	-.056	3	0	1	0	3	0	4
33		17	max	203.826	2	-.057	15	.151	1	0	10	0	1	0	15
34			min	-370.089	3	-.241	4	-.056	3	0	1	0	3	0	4
35		18	max	203.961	2	-.07	15	.151	1	0	10	0	1	0	15
36			min	-369.988	3	-.299	4	-.056	3	0	1	0	3	0	4
37		19	max	204.096	2	-.084	15	.151	1	0	10	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-369.887	3	-.356	4	-.056	3	0	1	0	3	0	4
39	M3	1	max	229.694	2	1.736	4	-.008	10	0	15	0	1	0	4
40			min	-221.587	3	.408	15	-.185	1	0	1	0	10	0	15
41		2	max	229.624	2	1.559	4	-.008	10	0	15	0	1	0	2
42			min	-221.64	3	.367	15	-.185	1	0	1	0	10	0	3
43		3	max	229.554	2	1.383	4	-.008	10	0	15	0	1	0	2
44			min	-221.692	3	.325	15	-.185	1	0	1	0	10	0	3
45		4	max	229.484	2	1.207	4	-.008	10	0	15	0	1	0	15
46			min	-221.745	3	.284	15	-.185	1	0	1	0	10	0	4
47		5	max	229.414	2	1.03	4	-.008	10	0	15	0	1	0	15
48			min	-221.797	3	.242	15	-.185	1	0	1	0	10	0	4
49		6	max	229.344	2	.854	4	-.008	10	0	15	0	1	0	15
50			min	-221.85	3	.201	15	-.185	1	0	1	0	10	0	4
51		7	max	229.274	2	.677	4	-.008	10	0	15	0	1	0	15
52			min	-221.902	3	.159	15	-.185	1	0	1	0	10	0	4
53		8	max	229.204	2	.501	4	-.008	10	0	15	0	1	0	15
54			min	-221.955	3	.118	15	-.185	1	0	1	0	10	-.001	4
55		9	max	229.134	2	.325	4	-.008	10	0	15	0	1	0	15
56			min	-222.007	3	.076	15	-.185	1	0	1	0	10	-.001	4
57		10	max	229.064	2	.148	4	-.008	10	0	15	0	1	0	15
58			min	-222.06	3	.035	15	-.185	1	0	1	0	10	-.001	4
59		11	max	228.994	2	.006	2	-.008	10	0	15	0	1	0	15
60			min	-222.112	3	-.054	3	-.185	1	0	1	0	10	-.001	4
61		12	max	228.924	2	-.048	15	-.008	10	0	15	0	1	0	15
62			min	-222.165	3	-.204	4	-.185	1	0	1	0	10	-.001	4
63		13	max	228.854	2	-.089	15	-.008	10	0	15	0	1	0	15
64			min	-222.217	3	-.381	4	-.185	1	0	1	0	10	-.001	4
65		14	max	228.784	2	-.131	15	-.008	10	0	15	0	1	0	15
66			min	-222.27	3	-.557	4	-.185	1	0	1	0	10	-.001	4
67		15	max	228.714	2	-.172	15	-.008	10	0	15	0	1	0	15
68			min	-222.322	3	-.734	4	-.185	1	0	1	0	10	0	4
69		16	max	228.644	2	-.214	15	-.008	10	0	15	0	1	0	15
70			min	-222.375	3	-.91	4	-.185	1	0	1	0	10	0	4
71		17	max	228.574	2	-.255	15	-.008	10	0	15	0	15	0	15
72			min	-222.427	3	-1.086	4	-.185	1	0	1	0	1	0	4
73		18	max	228.504	2	-.297	15	-.008	10	0	15	0	15	0	15
74			min	-222.48	3	-1.263	4	-.185	1	0	1	0	1	0	4
75		19	max	228.434	2	-.338	15	-.008	10	0	15	0	15	0	1
76			min	-222.532	3	-1.439	4	-.185	1	0	1	0	1	0	1
77	M4	1	max	268.521	1	0	1	-.03	10	0	1	0	3	0	1
78			min	10.614	15	0	1	-.695	1	0	1	0	2	0	1
79		2	max	268.586	1	0	1	-.03	10	0	1	0	15	0	1
80			min	10.633	15	0	1	-.695	1	0	1	0	1	0	1
81		3	max	268.65	1	0	1	-.03	10	0	1	0	15	0	1
82			min	10.653	15	0	1	-.695	1	0	1	0	1	0	1
83		4	max	268.715	1	0	1	-.03	10	0	1	0	15	0	1
84			min	10.672	15	0	1	-.695	1	0	1	0	1	0	1
85		5	max	268.78	1	0	1	-.03	10	0	1	0	15	0	1
86			min	10.692	15	0	1	-.695	1	0	1	0	1	0	1
87		6	max	268.844	1	0	1	-.03	10	0	1	0	15	0	1
88			min	10.711	15	0	1	-.695	1	0	1	0	1	0	1
89		7	max	268.909	1	0	1	-.03	10	0	1	0	15	0	1
90			min	10.731	15	0	1	-.695	1	0	1	0	1	0	1
91		8	max	268.974	1	0	1	-.03	10	0	1	0	15	0	1
92			min	10.75	15	0	1	-.695	1	0	1	0	1	0	1
93		9	max	269.039	1	0	1	-.03	10	0	1	0	15	0	1
94			min	10.77	15	0	1	-.695	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	269.103	1	0	1	-.03	10	0	1	0	15	0	1
96		min	10.789	15	0	1	-.695	1	0	1	0	1	0	1
97	11	max	269.168	1	0	1	-.03	10	0	1	0	15	0	1
98		min	10.809	15	0	1	-.695	1	0	1	0	1	0	1
99	12	max	269.233	1	0	1	-.03	10	0	1	0	15	0	1
100		min	10.828	15	0	1	-.695	1	0	1	0	1	0	1
101	13	max	269.297	1	0	1	-.03	10	0	1	0	15	0	1
102		min	10.848	15	0	1	-.695	1	0	1	0	1	0	1
103	14	max	269.362	1	0	1	-.03	10	0	1	0	15	0	1
104		min	10.868	15	0	1	-.695	1	0	1	0	1	0	1
105	15	max	269.427	1	0	1	-.03	10	0	1	0	15	0	1
106		min	10.887	15	0	1	-.695	1	0	1	0	1	0	1
107	16	max	269.492	1	0	1	-.03	10	0	1	0	15	0	1
108		min	10.907	15	0	1	-.695	1	0	1	0	1	0	1
109	17	max	269.556	1	0	1	-.03	10	0	1	0	15	0	1
110		min	10.926	15	0	1	-.695	1	0	1	-.001	1	0	1
111	18	max	269.621	1	0	1	-.03	10	0	1	0	15	0	1
112		min	10.946	15	0	1	-.695	1	0	1	-.001	1	0	1
113	19	max	269.686	1	0	1	-.03	10	0	1	0	15	0	1
114		min	10.965	15	0	1	-.695	1	0	1	-.001	1	0	1
115	M6	1	max	637.602	2	.68	.03	9	0	3	0	3	0	1
116		min	-1137.862	3	.16	15	-.25	3	0	2	0	2	0	1
117	2	max	637.737	2	.622	4	.03	9	0	3	0	3	0	15
118		min	-1137.761	3	.146	15	-.25	3	0	2	0	2	0	4
119	3	max	637.872	2	.565	4	.03	9	0	3	0	3	0	15
120		min	-1137.66	3	.133	15	-.25	3	0	2	0	2	0	4
121	4	max	638.007	2	.507	4	.03	9	0	3	0	3	0	15
122		min	-1137.559	3	.119	15	-.25	3	0	2	0	2	0	4
123	5	max	638.142	2	.45	4	.03	9	0	3	0	3	0	15
124		min	-1137.458	3	.099	12	-.25	3	0	2	0	2	0	4
125	6	max	638.276	2	.401	2	.03	9	0	3	0	3	0	15
126		min	-1137.356	3	.077	12	-.25	3	0	2	0	2	0	4
127	7	max	638.411	2	.357	2	.03	9	0	3	0	9	0	15
128		min	-1137.255	3	.054	12	-.25	3	0	2	0	2	0	4
129	8	max	638.546	2	.312	2	.03	9	0	3	0	9	0	12
130		min	-1137.154	3	.032	12	-.25	3	0	2	0	3	0	4
131	9	max	638.681	2	.267	2	.03	9	0	3	0	9	0	12
132		min	-1137.053	3	.002	3	-.25	3	0	2	0	3	0	4
133	10	max	638.816	2	.222	2	.03	9	0	3	0	9	0	12
134		min	-1136.952	3	-.032	3	-.25	3	0	2	0	3	0	2
135	11	max	638.951	2	.177	2	.03	9	0	3	0	9	0	12
136		min	-1136.851	3	-.065	3	-.25	3	0	2	0	3	0	2
137	12	max	639.086	2	.133	2	.03	9	0	3	0	9	0	12
138		min	-1136.75	3	-.099	3	-.25	3	0	2	0	3	0	2
139	13	max	639.22	2	.088	2	.03	9	0	3	0	9	0	12
140		min	-1136.648	3	-.133	3	-.25	3	0	2	0	3	0	2
141	14	max	639.355	2	.043	2	.03	9	0	3	0	9	0	12
142		min	-1136.547	3	-.166	3	-.25	3	0	2	0	3	0	2
143	15	max	639.49	2	-.002	2	.03	9	0	3	0	9	0	12
144		min	-1136.446	3	-.2	3	-.25	3	0	2	0	3	0	2
145	16	max	639.625	2	-.043	15	.03	9	0	3	0	9	0	3
146		min	-1136.345	3	-.233	3	-.25	3	0	2	0	3	0	2
147	17	max	639.76	2	-.057	15	.03	9	0	3	0	9	0	3
148		min	-1136.244	3	-.267	3	-.25	3	0	2	0	3	0	2
149	18	max	639.895	2	-.07	15	.03	9	0	3	0	9	0	3
150		min	-1136.143	3	-.301	3	-.25	3	0	2	0	3	0	2
151	19	max	640.03	2	-.084	15	.03	9	0	3	0	9	0	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1136.042	3	-.355	4	-.25	3	0	2	0	3	0	2
153	M7	1	max	766.468	2	1.739	4	.048	3	0	1	0	1	2
154		min	-654.73	3	.409	15	-.011	1	0	3	0	3	0	3
155		2	max	766.398	2	1.563	4	.048	3	0	1	0	1	2
156		min	-654.783	3	.367	15	-.011	1	0	3	0	3	0	3
157		3	max	766.328	2	1.386	4	.048	3	0	1	0	1	2
158		min	-654.835	3	.326	15	-.011	1	0	3	0	3	0	3
159		4	max	766.258	2	1.21	4	.048	3	0	1	0	1	2
160		min	-654.888	3	.284	15	-.011	1	0	3	0	3	0	3
161		5	max	766.188	2	1.033	4	.048	3	0	1	0	1	15
162		min	-654.94	3	.243	15	-.011	1	0	3	0	3	0	3
163		6	max	766.118	2	.857	4	.048	3	0	1	0	1	15
164		min	-654.993	3	.201	15	-.011	1	0	3	0	3	0	4
165		7	max	766.048	2	.681	4	.048	3	0	1	0	1	15
166		min	-655.045	3	.16	15	-.011	1	0	3	0	3	0	4
167		8	max	765.978	2	.504	4	.048	3	0	1	0	1	15
168		min	-655.098	3	.118	15	-.011	1	0	3	0	3	-.001	4
169		9	max	765.908	2	.342	2	.048	3	0	1	0	1	15
170		min	-655.15	3	.059	12	-.011	1	0	3	0	3	-.001	4
171		10	max	765.838	2	.205	2	.048	3	0	1	0	1	15
172		min	-655.203	3	-.026	3	-.011	1	0	3	0	3	-.001	4
173		11	max	765.768	2	.067	2	.048	3	0	1	0	1	15
174		min	-655.255	3	-.129	3	-.011	1	0	3	0	3	-.001	4
175		12	max	765.698	2	-.047	15	.048	3	0	1	0	1	15
176		min	-655.308	3	-.232	3	-.011	1	0	3	0	3	-.001	4
177		13	max	765.628	2	-.089	15	.048	3	0	1	0	1	15
178		min	-655.36	3	-.378	4	-.011	1	0	3	0	3	-.001	4
179		14	max	765.558	2	-.13	15	.048	3	0	1	0	1	15
180		min	-655.413	3	-.554	4	-.011	1	0	3	0	3	-.001	4
181		15	max	765.488	2	-.172	15	.048	3	0	1	0	1	15
182		min	-655.465	3	-.73	4	-.011	1	0	3	0	3	0	4
183		16	max	765.418	2	-.213	15	.048	3	0	1	0	1	15
184		min	-655.518	3	-.907	4	-.011	1	0	3	0	3	0	4
185		17	max	765.348	2	-.255	15	.048	3	0	1	0	1	15
186		min	-655.57	3	-1.083	4	-.011	1	0	3	0	3	0	4
187		18	max	765.278	2	-.296	15	.048	3	0	1	0	1	15
188		min	-655.623	3	-1.259	4	-.011	1	0	3	0	3	0	4
189		19	max	765.208	2	-.338	15	.048	3	0	1	0	1	1
190		min	-655.675	3	-1.436	4	-.011	1	0	3	0	3	0	1
191	M8	1	max	711.241	1	0	1	.243	1	0	1	0	2	1
192		min	23.74	15	0	1	-.735	3	0	1	0	3	0	1
193		2	max	711.306	1	0	1	.243	1	0	1	0	1	1
194		min	23.76	15	0	1	-.735	3	0	1	0	3	0	1
195		3	max	711.371	1	0	1	.243	1	0	1	0	1	1
196		min	23.779	15	0	1	-.735	3	0	1	0	3	0	1
197		4	max	711.435	1	0	1	.243	1	0	1	0	1	1
198		min	23.799	15	0	1	-.735	3	0	1	0	3	0	1
199		5	max	711.5	1	0	1	.243	1	0	1	0	1	1
200		min	23.818	15	0	1	-.735	3	0	1	0	3	0	1
201		6	max	711.565	1	0	1	.243	1	0	1	0	1	1
202		min	23.838	15	0	1	-.735	3	0	1	0	3	0	1
203		7	max	711.629	1	0	1	.243	1	0	1	0	1	1
204		min	23.857	15	0	1	-.735	3	0	1	0	3	0	1
205		8	max	711.694	1	0	1	.243	1	0	1	0	1	1
206		min	23.877	15	0	1	-.735	3	0	1	0	3	0	1
207		9	max	711.759	1	0	1	.243	1	0	1	0	1	1
208		min	23.896	15	0	1	-.735	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	711.823	1	0	1	.243	1	0	1	0	1	0	1
210		min	23.916	15	0	1	-.735	3	0	1	0	3	0	1
211	11	max	711.888	1	0	1	.243	1	0	1	0	1	0	1
212		min	23.935	15	0	1	-.735	3	0	1	0	3	0	1
213	12	max	711.953	1	0	1	.243	1	0	1	0	1	0	1
214		min	23.955	15	0	1	-.735	3	0	1	0	3	0	1
215	13	max	712.018	1	0	1	.243	1	0	1	0	1	0	1
216		min	23.974	15	0	1	-.735	3	0	1	0	3	0	1
217	14	max	712.082	1	0	1	.243	1	0	1	0	1	0	1
218		min	23.994	15	0	1	-.735	3	0	1	0	3	0	1
219	15	max	712.147	1	0	1	.243	1	0	1	0	1	0	1
220		min	24.013	15	0	1	-.735	3	0	1	0	3	0	1
221	16	max	712.212	1	0	1	.243	1	0	1	0	1	0	1
222		min	24.033	15	0	1	-.735	3	0	1	0	3	0	1
223	17	max	712.276	1	0	1	.243	1	0	1	0	1	0	1
224		min	24.052	15	0	1	-.735	3	0	1	-.001	3	0	1
225	18	max	712.341	1	0	1	.243	1	0	1	0	1	0	1
226		min	24.072	15	0	1	-.735	3	0	1	-.001	3	0	1
227	19	max	712.406	1	0	1	.243	1	0	1	0	1	0	1
228		min	24.092	15	0	1	-.735	3	0	1	-.001	3	0	1
229	M10	1	max	203.023	2	.678	.006	3	0	1	0	1	0	1
230		min	-302.046	3	.16	15	-.116	1	0	3	0	3	0	1
231	2	max	203.158	2	.621	4	.006	3	0	1	0	1	0	15
232		min	-301.944	3	.146	15	-.116	1	0	3	0	3	0	4
233	3	max	203.292	2	.563	4	.006	3	0	1	0	1	0	15
234		min	-301.843	3	.133	15	-.116	1	0	3	0	3	0	4
235	4	max	203.427	2	.506	4	.006	3	0	1	0	1	0	15
236		min	-301.742	3	.119	15	-.116	1	0	3	0	3	0	4
237	5	max	203.562	2	.448	4	.006	3	0	1	0	1	0	15
238		min	-301.641	3	.105	15	-.116	1	0	3	0	3	0	4
239	6	max	203.697	2	.391	4	.006	3	0	1	0	1	0	15
240		min	-301.54	3	.092	15	-.116	1	0	3	0	3	0	4
241	7	max	203.832	2	.333	4	.006	3	0	1	0	1	0	15
242		min	-301.439	3	.078	15	-.116	1	0	3	0	3	0	4
243	8	max	203.967	2	.276	4	.006	3	0	1	0	1	0	15
244		min	-301.338	3	.065	15	-.116	1	0	3	0	3	0	4
245	9	max	204.102	2	.218	4	.006	3	0	1	0	9	0	15
246		min	-301.236	3	.051	15	-.116	1	0	3	0	3	0	4
247	10	max	204.236	2	.161	4	.006	3	0	1	0	9	0	15
248		min	-301.135	3	.038	15	-.116	1	0	3	0	3	0	4
249	11	max	204.371	2	.11	2	.006	3	0	1	0	15	0	15
250		min	-301.034	3	.024	15	-.116	1	0	3	0	3	0	4
251	12	max	204.506	2	.066	2	.006	3	0	1	0	15	0	15
252		min	-300.933	3	.002	3	-.116	1	0	3	0	3	0	4
253	13	max	204.641	2	.021	2	.006	3	0	1	0	15	0	15
254		min	-300.832	3	-.032	3	-.116	1	0	3	0	3	0	4
255	14	max	204.776	2	-.016	15	.006	3	0	1	0	15	0	15
256		min	-300.731	3	-.069	4	-.116	1	0	3	0	3	0	4
257	15	max	204.911	2	-.03	15	.006	3	0	1	0	15	0	15
258		min	-300.63	3	-.126	4	-.116	1	0	3	0	3	0	4
259	16	max	205.046	2	-.043	15	.006	3	0	1	0	15	0	15
260		min	-300.528	3	-.184	4	-.116	1	0	3	0	3	0	4
261	17	max	205.181	2	-.057	15	.006	3	0	1	0	15	0	15
262		min	-300.427	3	-.241	4	-.116	1	0	3	0	3	0	4
263	18	max	205.315	2	-.07	15	.006	3	0	1	0	15	0	15
264		min	-300.326	3	-.299	4	-.116	1	0	3	0	3	0	4
265	19	max	205.45	2	-.084	15	.006	3	0	1	0	15	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	min	-300.225	3	-.356	4	-.116	1	0	3	0	1	0	4
267		max	229.216	2	1.736	4	.195	1	0	3	0	3	0	4
268		min	-222.4	3	.408	15	-.062	3	0	10	0	1	0	12
269		max	229.146	2	1.559	4	.195	1	0	3	0	3	0	2
270		min	-222.452	3	.367	15	-.062	3	0	10	0	1	0	3
271		max	229.076	2	1.383	4	.195	1	0	3	0	3	0	2
272		min	-222.505	3	.325	15	-.062	3	0	10	0	1	0	3
273		max	229.006	2	1.207	4	.195	1	0	3	0	3	0	15
274		min	-222.557	3	.284	15	-.062	3	0	10	0	1	0	4
275		max	228.936	2	1.03	4	.195	1	0	3	0	3	0	15
276		min	-222.61	3	.242	15	-.062	3	0	10	0	1	0	4
277		max	228.866	2	.854	4	.195	1	0	3	0	3	0	15
278		min	-222.662	3	.201	15	-.062	3	0	10	0	1	0	4
279		max	228.796	2	.677	4	.195	1	0	3	0	3	0	15
280		min	-222.715	3	.159	15	-.062	3	0	10	0	1	0	4
281		max	228.726	2	.501	4	.195	1	0	3	0	3	0	15
282		min	-222.767	3	.118	15	-.062	3	0	10	0	1	-.001	4
283		max	228.656	2	.325	4	.195	1	0	3	0	3	0	15
284		min	-222.82	3	.076	15	-.062	3	0	10	0	1	-.001	4
285		max	228.586	2	.148	4	.195	1	0	3	0	3	0	15
286		min	-222.872	3	.03	12	-.062	3	0	10	0	1	-.001	4
287		max	228.516	2	.006	2	.195	1	0	3	0	3	0	15
288		min	-222.925	3	-.062	3	-.062	3	0	10	0	1	-.001	4
289		max	228.446	2	-.048	15	.195	1	0	3	0	3	0	15
290		min	-222.977	3	-.204	4	-.062	3	0	10	0	1	-.001	4
291		max	228.376	2	-.089	15	.195	1	0	3	0	3	0	15
292		min	-223.03	3	-.381	4	-.062	3	0	10	0	1	-.001	4
293		max	228.306	2	-.131	15	.195	1	0	3	0	3	0	15
294		min	-223.082	3	-.557	4	-.062	3	0	10	0	1	-.001	4
295		max	228.236	2	-.172	15	.195	1	0	3	0	3	0	15
296		min	-223.135	3	-.734	4	-.062	3	0	10	0	1	0	4
297		max	228.166	2	-.214	15	.195	1	0	3	0	3	0	15
298		min	-223.187	3	-.91	4	-.062	3	0	10	0	2	0	4
299		max	228.096	2	-.255	15	.195	1	0	3	0	3	0	15
300		min	-223.24	3	-1.086	4	-.062	3	0	10	0	10	0	4
301		max	228.026	2	-.297	15	.195	1	0	3	0	3	0	15
302		min	-223.292	3	-1.263	4	-.062	3	0	10	0	10	0	4
303		max	227.956	2	-.338	15	.195	1	0	3	0	3	0	1
304		min	-223.345	3	-1.439	4	-.062	3	0	10	0	10	0	1
305		max	268.617	1	0	1	1.191	1	0	1	0	2	0	1
306		min	10.731	15	0	1	.031	10	0	1	0	3	0	1
307		max	268.682	1	0	1	1.191	1	0	1	0	1	0	1
308		min	10.751	15	0	1	.031	10	0	1	0	15	0	1
309		max	268.747	1	0	1	1.191	1	0	1	0	1	0	1
310		min	10.77	15	0	1	.031	10	0	1	0	15	0	1
311		max	268.811	1	0	1	1.191	1	0	1	0	1	0	1
312		min	10.79	15	0	1	.031	10	0	1	0	15	0	1
313		max	268.876	1	0	1	1.191	1	0	1	0	1	0	1
314		min	10.809	15	0	1	.031	10	0	1	0	10	0	1
315		max	268.941	1	0	1	1.191	1	0	1	0	1	0	1
316		min	10.829	15	0	1	.031	10	0	1	0	10	0	1
317		max	269.005	1	0	1	1.191	1	0	1	0	1	0	1
318		min	10.848	15	0	1	.031	10	0	1	0	10	0	1
319		max	269.07	1	0	1	1.191	1	0	1	0	1	0	1
320		min	10.868	15	0	1	.031	10	0	1	0	10	0	1
321		max	269.135	1	0	1	1.191	1	0	1	0	1	0	1
322		min	10.887	15	0	1	.031	10	0	1	0	10	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-89.33	1	-172.73	3	-26.423	1	0	2	-.05	1	0	3
381	M5	1	max	212.478	1	1121.689	3	0	2	0	.011	3	0	3
382		min	2.63	12	-713.397	2	-79.243	3	0	3	0	10	0	2
383		2	max	212.638	1	1121.518	3	0	2	0	0	9	.154	2
384		min	2.71	12	-713.626	2	-79.243	3	0	3	-.006	3	-.243	3
385		3	max	348.664	3	4.63	9	8.736	3	0	0	9	.307	2
386		min	-86.918	2	-103.072	2	-.284	9	0	1	-.023	3	-.481	3
387		4	max	348.785	3	4.439	9	8.736	3	0	0	9	.329	2
388		min	-86.758	2	-103.3	2	-.284	9	0	1	-.021	3	-.473	3
389		5	max	348.905	3	4.248	9	8.736	3	0	0	9	.351	2
390		min	-86.598	2	-103.529	2	-.284	9	0	1	-.019	3	-.465	3
391		6	max	349.025	3	4.058	9	8.736	3	0	0	9	.374	2
392		min	-86.437	2	-103.758	2	-.284	9	0	1	-.017	3	-.457	3
393		7	max	349.145	3	3.867	9	8.736	3	0	0	9	.396	2
394		min	-86.277	2	-103.987	2	-.284	9	0	1	-.015	3	-.449	3
395		8	max	349.265	3	3.676	9	8.736	3	0	0	9	.419	2
396		min	-86.117	2	-104.215	2	-.284	9	0	1	-.013	3	-.441	3
397		9	max	349.385	3	3.486	9	8.736	3	0	0	1	.442	2
398		min	-85.957	2	-104.444	2	-.284	9	0	1	-.011	3	-.433	3
399		10	max	349.505	3	3.295	9	8.736	3	0	0	2	.464	2
400		min	-85.797	2	-104.673	2	-.284	9	0	1	-.009	3	-.425	3
401		11	max	349.625	3	3.105	9	8.736	3	0	0	2	.487	2
402		min	-85.637	2	-104.902	2	-.284	9	0	1	-.008	3	-.417	3
403		12	max	349.746	3	2.914	9	8.736	3	0	0	2	.51	2
404		min	-85.476	2	-105.13	2	-.284	9	0	1	-.006	3	-.408	3
405		13	max	349.866	3	2.723	9	8.736	3	0	0	2	.533	2
406		min	-85.316	2	-105.359	2	-.284	9	0	1	-.004	3	-.4	3
407		14	max	349.986	3	2.533	9	8.736	3	0	0	2	.556	2
408		min	-85.156	2	-105.588	2	-.284	9	0	1	-.002	3	-.392	3
409		15	max	350.106	3	2.342	9	8.736	3	0	0	3	.578	2
410		min	-84.996	2	-105.816	2	-.284	9	0	1	0	9	-.384	3
411		16	max	274.967	2	572.54	2	8.717	3	0	.001	3	.595	2
412		min	4.05	15	-618.402	3	-.283	9	0	2	0	9	-.37	3
413		17	max	275.127	2	572.311	2	8.717	3	0	.003	3	.471	2
414		min	4.098	15	-618.573	3	-.283	9	0	2	0	9	-.236	3
415		18	max	-6.004	12	1092.919	2	7.975	3	0	.005	3	.236	2
416		min	-212.639	1	-544.402	3	-.062	1	0	1	0	9	-.118	3
417		19	max	-5.924	12	1092.69	2	7.975	3	0	.007	3	0	3
418		min	-212.479	1	-544.574	3	-.062	1	0	1	0	9	0	2
419	M9	1	max	89.128	1	347.687	3	84.662	3	0	-.002	10	0	2
420		min	4.106	15	-223.307	2	1.048	10	0	2	-.05	1	0	3
421		2	max	89.288	1	347.516	3	84.662	3	0	-.001	12	.049	2
422		min	4.154	15	-223.536	2	1.048	10	0	2	-.044	1	-.076	3
423		3	max	118.238	3	4.58	9	25.112	1	0	.016	3	.096	2
424		min	-22.326	2	-30.232	2	-2.473	3	0	12	-.038	1	-.149	3
425		4	max	118.358	3	4.39	9	25.112	1	0	.015	3	.103	2
426		min	-22.166	2	-30.461	2	-2.473	3	0	12	-.033	1	-.148	3
427		5	max	118.478	3	4.199	9	25.112	1	0	.015	3	.11	2
428		min	-22.006	2	-30.689	2	-2.473	3	0	12	-.027	1	-.146	3
429		6	max	118.598	3	4.008	9	25.112	1	0	.014	3	.116	2
430		min	-21.846	2	-30.918	2	-2.473	3	0	12	-.022	1	-.144	3
431		7	max	118.718	3	3.818	9	25.112	1	0	.014	3	.123	2
432		min	-21.685	2	-31.147	2	-2.473	3	0	12	-.016	1	-.142	3
433		8	max	118.838	3	3.627	9	25.112	1	0	.013	3	.13	2
434		min	-21.525	2	-31.375	2	-2.473	3	0	12	-.011	1	-.14	3
435		9	max	118.959	3	3.436	9	25.112	1	0	.012	3	.137	2
436		min	-21.365	2	-31.604	2	-2.473	3	0	12	-.005	1	-.138	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	119.079	3	3.246	9	25.112	1	0	1	.012	3	.144	2
438		min	-21.205	2	-31.833	2	-2.473	3	0	12	0	1	-.136	3
439	11	max	119.199	3	3.055	9	25.112	1	0	1	.011	3	.151	2
440		min	-21.045	2	-32.062	2	-2.473	3	0	12	0	10	-.133	3
441	12	max	119.319	3	2.865	9	25.112	1	0	1	.011	1	.157	2
442		min	-20.885	2	-32.29	2	-2.473	3	0	12	0	10	-.131	3
443	13	max	119.439	3	2.674	9	25.112	1	0	1	.016	1	.165	2
444		min	-20.724	2	-32.519	2	-2.473	3	0	12	0	10	-.129	3
445	14	max	119.559	3	2.483	9	25.112	1	0	1	.022	1	.172	2
446		min	-20.564	2	-32.748	2	-2.473	3	0	12	0	10	-.127	3
447	15	max	119.679	3	2.293	9	25.112	1	0	1	.027	1	.179	2
448		min	-20.404	2	-32.977	2	-2.473	3	0	12	.001	10	-.125	3
449	16	max	87.73	2	167.881	2	25.277	1	0	15	.033	1	.184	2
450		min	1.905	15	-208.45	3	-2.538	3	0	3	.001	10	-.12	3
451	17	max	87.89	2	167.652	2	25.277	1	0	15	.038	1	.148	2
452		min	1.953	15	-208.621	3	-2.538	3	0	3	.002	10	-.075	3
453	18	max	-4.154	15	341.393	2	26.489	1	0	2	.044	1	.074	2
454		min	-89.287	1	-172.547	3	-2.019	3	0	3	.002	10	-.038	3
455	19	max	-4.105	15	341.164	2	26.489	1	0	2	.05	1	0	2
456		min	-89.127	1	-172.719	3	-2.019	3	0	3	.002	10	0	3
457	M13	1	max	84.655	3	223.198	2	-4.106	15	0	.05	1	0	2
458		min	1.048	10	-347.737	3	-89.121	1	0	3	.002	10	0	3
459	2	max	84.655	3	158.823	2	-3.125	15	0	2	.015	3	.149	3
460		min	1.048	10	-246.791	3	-67.402	1	0	3	-.002	10	-.095	2
461	3	max	84.655	3	94.447	2	-2.145	15	0	2	.011	3	.247	3
462		min	1.048	10	-145.845	3	-45.684	1	0	3	-.018	1	-.159	2
463	4	max	84.655	3	30.072	2	-.979	10	0	2	.007	3	.294	3
464		min	1.048	10	-44.899	3	-23.965	1	0	3	-.035	1	-.19	2
465	5	max	84.655	3	56.047	3	2.013	2	0	2	.004	3	.292	3
466		min	1.048	10	-34.304	2	-5.328	3	0	3	-.042	1	-.189	2
467	6	max	84.655	3	156.993	3	19.473	1	0	2	.002	3	.238	3
468		min	1.048	10	-98.679	2	-3.901	3	0	3	-.037	1	-.156	2
469	7	max	84.655	3	257.939	3	41.191	1	0	2	0	3	.135	3
470		min	1.048	10	-163.055	2	-2.475	3	0	3	-.022	1	-.09	2
471	8	max	84.655	3	358.885	3	62.91	1	0	2	.006	2	.008	1
472		min	1.048	10	-227.43	2	-1.048	3	0	3	0	3	-.02	3
473	9	max	84.655	3	459.831	3	84.629	1	0	2	.041	1	.137	2
474		min	1.048	10	-291.805	2	.379	3	0	3	-.001	3	-.224	3
475	10	max	84.655	3	560.777	3	106.347	1	0	2	.089	1	.299	2
476		min	1.048	10	-356.181	2	1.542	12	0	3	-.011	3	-.479	3
477	11	max	25.654	1	291.805	2	.391	3	0	3	.041	1	.137	2
478		min	1.048	10	-459.831	3	-84.426	1	0	2	-.012	3	-.224	3
479	12	max	25.654	1	227.43	2	1.818	3	0	3	.006	2	.008	1
480		min	1.048	10	-358.885	3	-62.707	1	0	2	-.011	3	-.02	3
481	13	max	25.654	1	163.055	2	3.245	3	0	3	0	10	.135	3
482		min	1.048	10	-257.939	3	-40.988	1	0	2	-.022	1	-.09	2
483	14	max	25.654	1	98.679	2	4.672	3	0	3	-.002	15	.238	3
484		min	1.048	10	-156.993	3	-19.27	1	0	2	-.037	1	-.156	2
485	15	max	25.654	1	34.304	2	6.098	3	0	3	-.002	15	.292	3
486		min	1.048	10	-56.047	3	-2.013	2	0	2	-.041	1	-.189	2
487	16	max	25.654	1	44.899	3	24.168	1	0	3	-.001	12	.294	3
488		min	1.048	10	-30.072	2	.979	10	0	2	-.035	1	-.19	2
489	17	max	25.654	1	145.845	3	45.887	1	0	3	.002	3	.247	3
490		min	1.048	10	-94.447	2	2.16	15	0	2	-.017	1	-.159	2
491	18	max	25.654	1	246.791	3	67.605	1	0	3	.011	1	.149	3
492		min	1.048	10	-158.823	2	3.14	15	0	2	-.002	10	-.095	2
493	19	max	25.654	1	347.737	3	89.324	1	0	3	.05	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	1.048	10	-223.198	2	4.121	15	0	2	.002	10	0	3
495		max	2.023	3	341.306	2	-4.105	15	0	3	.05	1	0	2
496		min	-26.433	1	-172.752	3	-89.134	1	0	2	.002	10	0	3
497		max	2.023	3	242.602	2	-3.125	15	0	3	.011	1	.074	3
498		min	-26.433	1	-123.298	3	-67.415	1	0	2	-.002	10	-.146	2
499		max	2.023	3	143.899	2	-2.145	15	0	3	0	3	.123	3
500		min	-26.433	1	-73.844	3	-45.697	1	0	2	-.018	1	-.243	2
501		max	2.023	3	45.195	2	-1	10	0	3	-.002	15	.148	3
502		min	-26.433	1	-24.39	3	-23.978	1	0	2	-.035	1	-.29	2
503		max	2.023	3	25.063	3	1.975	2	0	3	-.002	15	.148	3
504	M15	min	-26.433	1	-53.508	2	-3.568	3	0	2	-.042	1	-.288	2
505		max	2.023	3	74.517	3	19.459	1	0	3	-.002	15	.123	3
506		min	-26.433	1	-152.212	2	-2.141	3	0	2	-.037	1	-.236	2
507		max	2.023	3	123.971	3	41.178	1	0	3	0	10	.073	3
508		min	-26.433	1	-250.915	2	-.714	3	0	2	-.022	1	-.136	2
509		max	2.023	3	173.425	3	62.897	1	0	3	.006	2	.015	2
510		min	-26.433	1	-349.619	2	.696	12	0	2	-.007	3	-.001	3
511		max	2.023	3	222.879	3	84.615	1	0	3	.041	1	.214	2
512		min	-26.433	1	-448.322	2	1.647	12	0	2	-.006	3	-.1	3
513		max	-1.079	10	-7.773	15	106.334	1	0	15	.089	1	.463	2
514	M14	min	-26.433	1	-547.026	2	-4.788	3	0	2	-.005	3	-.224	3
515		max	-1.079	10	448.322	2	-2.375	12	0	2	.041	1	.214	2
516		min	-26.369	1	-222.879	3	-84.413	1	0	3	0	3	-.1	3
517		max	-1.079	10	349.619	2	-1.424	12	0	2	.006	2	.015	2
518		min	-26.369	1	-173.425	3	-62.694	1	0	3	-.002	3	-.001	3
519		max	-1.079	10	250.915	2	-.473	12	0	2	0	10	.073	3
520		min	-26.369	1	-123.971	3	-40.975	1	0	3	-.022	1	-.136	2
521		max	-1.079	10	152.212	2	.919	3	0	2	-.001	12	.123	3
522		min	-26.369	1	-74.517	3	-19.257	1	0	3	-.037	1	-.236	2
523		max	-1.079	10	53.508	2	2.88	9	0	2	-.001	12	.148	3
524	M13	min	-26.369	1	-25.063	3	-1.975	2	0	3	-.041	1	-.288	2
525		max	-1.079	10	24.391	3	24.181	1	0	2	0	3	.148	3
526		min	-26.369	1	-45.195	2	1	10	0	3	-.035	1	-.29	2
527		max	-1.079	10	73.844	3	45.899	1	0	2	.002	3	.123	3
528		min	-26.369	1	-143.899	2	2.159	15	0	3	-.017	1	-.243	2
529		max	-1.079	10	123.298	3	67.618	1	0	2	.011	1	.074	3
530		min	-26.369	1	-242.602	2	3.139	15	0	3	-.002	10	-.146	2
531		max	-1.079	10	172.752	3	89.337	1	0	2	.05	1	0	2
532		min	-26.369	1	-341.306	2	4.12	15	0	3	.002	10	0	3
533		max	0	1	.939	3	.114	3	0	1	0	1	0	1
534	M12	min	-111.709	3	0	1	0	1	0	3	0	3	0	1
535		max	0	1	.834	3	.114	3	0	1	0	1	0	1
536		min	-111.785	3	0	1	0	1	0	3	0	3	0	3
537		max	0	1	.73	3	.114	3	0	1	0	1	0	1
538		min	-111.86	3	0	1	0	1	0	3	0	3	0	3
539		max	0	1	.626	3	.114	3	0	1	0	1	0	1
540		min	-111.936	3	0	1	0	1	0	3	0	3	0	3
541		max	0	1	.522	3	.114	3	0	1	0	1	0	1
542		min	-112.011	3	0	1	0	1	0	3	0	3	0	3
543		max	0	1	.417	3	.114	3	0	1	0	1	0	1
544	M11	min	-112.087	3	0	1	0	1	0	3	0	3	-.001	3
545		max	0	1	.313	3	.114	3	0	1	0	3	0	1
546		min	-112.162	3	0	1	0	1	0	3	0	1	-.001	3
547		max	0	1	.209	3	.114	3	0	1	0	3	0	1
548		min	-112.238	3	0	1	0	1	0	3	0	1	-.001	3
549		max	0	1	.104	3	.114	3	0	1	0	3	0	1
550		min	-112.314	3	0	1	0	1	0	3	0	1	-.001	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.114	3	0	1	0	3	0	1
552		min	-112.389	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.114	3	0	1	0	3	0	1
554		min	-112.465	3	-.104	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.114	3	0	1	0	3	0	1
556		min	-112.54	3	-.209	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.114	3	0	1	0	3	0	1
558		min	-112.616	3	-.313	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.114	3	0	1	0	3	0	1
560		min	-112.691	3	-.417	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.114	3	0	1	0	3	0	1
562		min	-112.767	3	-.522	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.114	3	0	1	0	3	0	1
564		min	-112.842	3	-.626	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.114	3	0	1	0	3	0	1
566		min	-112.918	3	-.73	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.114	3	0	1	0	3	0	1
568		min	-112.993	3	-.834	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.114	3	0	1	0	3	0	1
570		min	-113.069	3	-.939	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	1.606	.032	1	0	3	0	3	0	1
572		min	-111.511	3	0	2	-.047	3	0	1	0	1	0	1
573	2	max	0	2	1.428	4	.032	1	0	3	0	3	0	2
574		min	-111.436	3	0	2	-.047	3	0	1	0	1	0	4
575	3	max	0	2	1.249	4	.032	1	0	3	0	3	0	2
576		min	-111.36	3	0	2	-.047	3	0	1	0	1	0	4
577	4	max	0	2	1.071	4	.032	1	0	3	0	3	0	2
578		min	-111.284	3	0	2	-.047	3	0	1	0	1	-.001	4
579	5	max	0	2	.892	4	.032	1	0	3	0	3	0	2
580		min	-111.209	3	0	2	-.047	3	0	1	0	1	-.002	4
581	6	max	0	2	.714	4	.032	1	0	3	0	3	0	2
582		min	-111.133	3	0	2	-.047	3	0	1	0	1	-.002	4
583	7	max	0	2	.535	4	.032	1	0	3	0	3	0	2
584		min	-111.058	3	0	2	-.047	3	0	1	0	1	-.002	4
585	8	max	0	2	.357	4	.032	1	0	3	0	3	0	2
586		min	-110.982	3	0	2	-.047	3	0	1	0	1	-.002	4
587	9	max	0	2	.178	4	.032	1	0	3	0	3	0	2
588		min	-110.907	3	0	2	-.047	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.032	1	0	3	0	3	0	2
590		min	-110.831	3	0	1	-.047	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.032	1	0	3	0	3	0	2
592		min	-110.756	3	-.178	4	-.047	3	0	1	0	1	-.002	4
593	12	max	0	2	0	2	.032	1	0	3	0	3	0	2
594		min	-110.68	3	-.357	4	-.047	3	0	1	0	1	-.002	4
595	13	max	.086	13	0	2	.032	1	0	3	0	1	0	2
596		min	-110.605	3	-.535	4	-.047	3	0	1	0	3	-.002	4
597	14	max	.19	13	0	2	.032	1	0	3	0	1	0	2
598		min	-110.529	3	-.714	4	-.047	3	0	1	0	3	-.002	4
599	15	max	.294	13	0	2	.032	1	0	3	0	1	0	2
600		min	-110.454	3	-.892	4	-.047	3	0	1	0	3	-.002	4
601	16	max	.398	13	0	2	.032	1	0	3	0	1	0	2
602		min	-110.378	3	-1.071	4	-.047	3	0	1	0	3	-.001	4
603	17	max	.502	13	0	2	.032	1	0	3	0	1	0	2
604		min	-110.303	3	-1.249	4	-.047	3	0	1	0	3	0	4
605	18	max	.606	13	0	2	.032	1	0	3	0	1	0	2
606		min	-110.227	3	-1.428	4	-.047	3	0	1	0	3	0	4
607	19	max	.714	4	0	2	.032	1	0	3	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-110.152	3	-1.606	4	-.047	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.011	2	.004	1	-1.629e-5	10	NC	3	NC	2	
2			min	-.004	3	-.011	3	-.002	3	-4.193e-4	1	3918.474	2	9529.987	1	
3			2	max	.002	2	.01	2	.004	1	-1.551e-5	10	NC	3	NC	1
4				min	-.004	3	-.011	3	-.002	3	-4.004e-4	1	4285.139	2	NC	1
5			3	max	.002	2	.009	2	.004	1	-1.472e-5	10	NC	3	NC	1
6				min	-.003	3	-.01	3	-.002	3	-3.815e-4	1	4722.761	2	NC	1
7			4	max	.002	2	.008	2	.004	1	-1.394e-5	10	NC	1	NC	1
8				min	-.003	3	-.01	3	-.002	3	-3.625e-4	1	5248.652	2	NC	1
9			5	max	.002	2	.007	2	.003	1	-1.316e-5	10	NC	1	NC	1
10				min	-.003	3	-.009	3	-.002	3	-3.436e-4	1	5885.93	2	NC	1
11		6	max	.002	2	.006	2	.003	1	-1.238e-5	10	NC	1	NC	1	
12			min	-.003	3	-.009	3	-.001	3	-3.247e-4	1	6665.97	2	NC	1	
13		7	max	.001	2	.006	2	.003	1	-1.16e-5	10	NC	1	NC	1	
14			min	-.003	3	-.008	3	-.001	3	-3.057e-4	1	7632.156	2	NC	1	
15		8	max	.001	2	.005	2	.002	1	-1.082e-5	10	NC	1	NC	1	
16			min	-.002	3	-.008	3	-.001	3	-2.868e-4	1	8845.8	2	NC	1	
17		9	max	.001	2	.004	2	.002	1	-1.004e-5	10	NC	1	NC	1	
18			min	-.002	3	-.007	3	0	3	-2.678e-4	1	NC	1	NC	1	
19		10	max	.001	2	.003	2	.002	1	-9.26e-6	10	NC	1	NC	1	
20			min	-.002	3	-.007	3	0	3	-2.489e-4	1	NC	1	NC	1	
21		11	max	0	2	.003	2	.001	1	-8.479e-6	10	NC	1	NC	1	
22			min	-.002	3	-.006	3	0	3	-2.3e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	.001	1	-7.699e-6	10	NC	1	NC	1	
24			min	-.002	3	-.005	3	0	3	-2.11e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	1	-6.918e-6	10	NC	1	NC	1	
26			min	-.001	3	-.005	3	0	3	-1.921e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	1	-6.138e-6	10	NC	1	NC	1	
28			min	-.001	3	-.004	3	0	3	-1.732e-4	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	1	-5.357e-6	10	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-1.542e-4	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	1	-4.576e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-1.353e-4	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	1	-3.796e-6	10	NC	1	NC	1	
34			min	0	3	-.002	3	0	3	-1.164e-4	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	1	-3.015e-6	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-9.743e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-2.234e-6	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-7.849e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	3.764e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	1.076e-6	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	4.741e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	1.599e-6	10	NC	1	NC	1
43			3	max	0	3	0	2	0	12	5.719e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	2.122e-6	10	NC	1	NC	1
45			4	max	0	3	0	2	0	3	6.697e-5	1	NC	1	NC	1
46				min	0	2	-.003	3	0	1	2.645e-6	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	7.674e-5	1	NC	1	NC	1
48				min	0	2	-.004	3	0	1	3.168e-6	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	8.652e-5	1	NC	1	NC	1
50				min	0	2	-.005	3	0	9	3.691e-6	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	9.63e-5	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	9	4.214e-6	10	NC	1	NC	1
53	8	max	0	3	.001	2	0	3	1.061e-4	1	NC	1	NC	1
54		min	-.001	2	-.006	3	0	9	4.737e-6	10	NC	1	NC	1
55	9	max	.001	3	.001	2	0	3	1.159e-4	1	NC	1	NC	1
56		min	-.001	2	-.007	3	0	9	5.26e-6	10	NC	1	NC	1
57	10	max	.001	3	.002	2	0	3	1.256e-4	1	NC	1	NC	1
58		min	-.001	2	-.007	3	0	15	5.783e-6	10	NC	1	NC	1
59	11	max	.001	3	.002	2	0	1	1.354e-4	1	NC	1	NC	1
60		min	-.001	2	-.008	3	0	15	6.306e-6	10	NC	1	NC	1
61	12	max	.002	3	.003	2	0	1	1.452e-4	1	NC	1	NC	1
62		min	-.002	2	-.008	3	0	15	6.829e-6	10	NC	1	NC	1
63	13	max	.002	3	.004	2	.001	1	1.55e-4	1	NC	1	NC	1
64		min	-.002	2	-.008	3	0	15	7.304e-6	15	NC	1	NC	1
65	14	max	.002	3	.005	2	.001	1	1.647e-4	1	NC	1	NC	1
66		min	-.002	2	-.009	3	0	15	7.748e-6	15	NC	1	NC	1
67	15	max	.002	3	.005	2	.002	1	1.745e-4	1	NC	1	NC	1
68		min	-.002	2	-.009	3	0	15	8.192e-6	15	8441.435	2	NC	1
69	16	max	.002	3	.006	2	.002	1	1.843e-4	1	NC	1	NC	1
70		min	-.002	2	-.009	3	0	15	8.637e-6	15	7173.344	2	NC	1
71	17	max	.002	3	.007	2	.002	1	1.941e-4	1	NC	1	NC	1
72		min	-.002	2	-.009	3	0	15	9.081e-6	15	6188.35	2	NC	1
73	18	max	.002	3	.009	2	.003	1	2.038e-4	1	NC	1	NC	1
74		min	-.002	2	-.009	3	0	15	9.526e-6	15	5414.852	2	NC	1
75	19	max	.003	3	.01	2	.003	1	2.136e-4	1	NC	3	NC	1
76		min	-.003	2	-.009	3	0	15	9.97e-6	15	4802.213	2	NC	1
77	M4	1	max	.001	1	.013	2	15	-9.638e-6	12	NC	1	NC	2
78		min	0	15	-.011	3	-.002	1	-3.195e-4	1	NC	1	8626.778	1
79	2	max	.001	1	.012	2	0	15	-9.638e-6	12	NC	1	NC	2
80		min	0	15	-.011	3	-.002	1	-3.195e-4	1	NC	1	9410.889	1
81	3	max	.001	1	.011	2	0	15	-9.638e-6	12	NC	1	NC	1
82		min	0	15	-.01	3	-.002	1	-3.195e-4	1	NC	1	NC	1
83	4	max	.001	1	.01	2	0	15	-9.638e-6	12	NC	1	NC	1
84		min	0	15	-.009	3	-.002	1	-3.195e-4	1	NC	1	NC	1
85	5	max	0	1	.01	2	0	15	-9.638e-6	12	NC	1	NC	1
86		min	0	15	-.009	3	-.002	1	-3.195e-4	1	NC	1	NC	1
87	6	max	0	1	.009	2	0	15	-9.638e-6	12	NC	1	NC	1
88		min	0	15	-.008	3	-.001	1	-3.195e-4	1	NC	1	NC	1
89	7	max	0	1	.008	2	0	15	-9.638e-6	12	NC	1	NC	1
90		min	0	15	-.007	3	-.001	1	-3.195e-4	1	NC	1	NC	1
91	8	max	0	1	.008	2	0	15	-9.638e-6	12	NC	1	NC	1
92		min	0	15	-.007	3	-.001	1	-3.195e-4	1	NC	1	NC	1
93	9	max	0	1	.007	2	0	15	-9.638e-6	12	NC	1	NC	1
94		min	0	15	-.006	3	0	1	-3.195e-4	1	NC	1	NC	1
95	10	max	0	1	.006	2	0	15	-9.638e-6	12	NC	1	NC	1
96		min	0	15	-.006	3	0	1	-3.195e-4	1	NC	1	NC	1
97	11	max	0	1	.006	2	0	15	-9.638e-6	12	NC	1	NC	1
98		min	0	15	-.005	3	0	1	-3.195e-4	1	NC	1	NC	1
99	12	max	0	1	.005	2	0	15	-9.638e-6	12	NC	1	NC	1
100		min	0	15	-.004	3	0	1	-3.195e-4	1	NC	1	NC	1
101	13	max	0	1	.004	2	0	15	-9.638e-6	12	NC	1	NC	1
102		min	0	15	-.004	3	0	1	-3.195e-4	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-9.638e-6	12	NC	1	NC	1
104		min	0	15	-.003	3	0	1	-3.195e-4	1	NC	1	NC	1
105	15	max	0	1	.003	2	0	15	-9.638e-6	12	NC	1	NC	1
106		min	0	15	-.002	3	0	1	-3.195e-4	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-9.638e-6	12	NC	1	NC	1
108		min	0	15	-.002	3	0	1	-3.195e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-9.638e-6	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-3.195e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-9.638e-6	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-3.195e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-9.638e-6	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-3.195e-4	1	NC	1	NC	1
115	M6	1	max	.007	2	.036	2	.001	9	5.24e-4	3	NC	3	NC	1
116			min	-.012	3	-.034	3	-.007	3	-2.057e-7	1	1194.202	2	6282.727	3
117		2	max	.006	2	.033	2	.001	9	5.068e-4	3	NC	3	NC	1
118			min	-.011	3	-.033	3	-.006	3	-2.185e-6	1	1279.162	2	6640.973	3
119		3	max	.006	2	.031	2	.001	9	4.897e-4	3	NC	3	NC	1
120			min	-.011	3	-.031	3	-.006	3	-4.165e-6	1	1376.682	2	7070.571	3
121		4	max	.006	2	.029	2	.001	9	4.725e-4	3	NC	3	NC	1
122			min	-.01	3	-.029	3	-.006	3	-6.145e-6	1	1489.273	2	7586.143	3
123		5	max	.005	2	.026	2	.001	9	4.554e-4	3	NC	3	NC	1
124			min	-.009	3	-.027	3	-.005	3	-8.124e-6	1	1620.167	2	8207.203	3
125		6	max	.005	2	.024	2	0	9	4.382e-4	3	NC	3	NC	1
126			min	-.009	3	-.025	3	-.005	3	-1.01e-5	1	1773.597	2	8960.102	3
127		7	max	.004	2	.022	2	0	9	4.211e-4	3	NC	3	NC	1
128			min	-.008	3	-.023	3	-.004	3	-1.208e-5	1	1955.206	2	9881.01	3
129		8	max	.004	2	.02	2	0	9	4.039e-4	3	NC	3	NC	1
130			min	-.007	3	-.022	3	-.004	3	-1.406e-5	1	2172.69	2	NC	1
131		9	max	.004	2	.017	2	0	1	3.868e-4	3	NC	3	NC	1
132			min	-.007	3	-.02	3	-.003	3	-1.604e-5	1	2436.821	2	NC	1
133		10	max	.003	2	.015	2	0	1	3.696e-4	3	NC	3	NC	1
134			min	-.006	3	-.018	3	-.003	3	-1.802e-5	1	2763.146	2	NC	1
135		11	max	.003	2	.013	2	0	1	3.525e-4	3	NC	3	NC	1
136			min	-.005	3	-.016	3	-.003	3	-2.e-5	1	3174.974	2	NC	1
137		12	max	.003	2	.011	2	0	1	3.353e-4	3	NC	3	NC	1
138			min	-.005	3	-.014	3	-.002	3	-2.198e-5	1	3708.909	2	NC	1
139		13	max	.002	2	.01	2	0	1	3.182e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.002	3	-2.396e-5	1	4425.916	2	NC	1
141		14	max	.002	2	.008	2	0	1	3.01e-4	3	NC	1	NC	1
142			min	-.003	3	-.01	3	-.001	3	-2.594e-5	1	5435.677	2	NC	1
143		15	max	.001	2	.006	2	0	1	2.839e-4	3	NC	1	NC	1
144			min	-.003	3	-.008	3	-.001	3	-2.792e-5	1	6957.464	2	NC	1
145		16	max	.001	2	.004	2	0	1	2.667e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-2.99e-5	1	9502.76	2	NC	1
147		17	max	0	2	.003	2	0	1	2.496e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-3.188e-5	1	NC	1	NC	1
149		18	max	0	2	.001	2	0	1	2.324e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-3.386e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.152e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.584e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.707e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.025e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.511e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-7.62e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.316e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.988e-5	3	NC	1	NC	1
159		4	max	.001	3	.004	2	.001	3	1.12e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.357e-5	3	NC	1	NC	1
161		5	max	.002	3	.006	2	.002	3	9.24e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	0	2	8129.703	2	NC	1
163		6	max	.002	3	.007	2	.002	3	2.906e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	0	2	6506.65	2	NC	1
165		7	max	.002	3	.009	2	.002	3	5.537e-5	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.012	3	0	1	-1.125e-7	13	5399.098	2	NC	1
167		8	max	.003	3	.01	2	.003	3	8.168e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-1.319e-6	9	4589.053	2	NC	1
169		9	max	.003	3	.012	2	.003	3	1.08e-4	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-3.211e-6	9	3967.812	2	NC	1
171		10	max	.004	3	.013	2	.003	3	1.343e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	0	1	-5.103e-6	9	3475.07	2	NC	1
173		11	max	.004	3	.015	2	.003	3	1.606e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	0	1	-6.994e-6	9	3074.595	2	NC	1
175		12	max	.005	3	.017	2	.003	3	1.869e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	0	1	-8.886e-6	9	2743.195	2	NC	1
177		13	max	.005	3	.019	2	.003	3	2.132e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	0	1	-1.078e-5	9	2465.238	2	NC	1
179		14	max	.005	3	.021	2	.003	3	2.396e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	0	1	-1.267e-5	9	2229.722	2	NC	1
181		15	max	.006	3	.023	2	.003	3	2.659e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	0	1	-1.456e-5	9	2028.617	2	NC	1
183		16	max	.006	3	.025	2	.003	3	2.922e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	0	1	-1.645e-5	9	1855.884	2	NC	1
185		17	max	.007	3	.027	2	.003	3	3.185e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	0	1	-1.834e-5	9	1706.872	2	NC	1
187		18	max	.007	3	.029	2	.003	3	3.448e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	0	1	-2.024e-5	9	1577.926	2	NC	1
189		19	max	.007	3	.031	2	.003	3	3.711e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	0	9	-2.213e-5	9	1466.13	2	NC	1
191	M8	1	max	.003	1	.041	2	0	1	-1.172e-7	10	NC	1	NC	1
192			min	0	15	-.034	3	-.002	3	-2.793e-4	3	NC	1	8314.499	3
193		2	max	.003	1	.039	2	0	1	-1.172e-7	10	NC	1	NC	1
194			min	0	15	-.032	3	-.002	3	-2.793e-4	3	NC	1	9065.495	3
195		3	max	.003	1	.037	2	0	1	-1.172e-7	10	NC	1	NC	1
196			min	0	15	-.03	3	-.002	3	-2.793e-4	3	NC	1	9959.536	3
197		4	max	.003	1	.034	2	0	1	-1.172e-7	10	NC	1	NC	1
198			min	0	15	-.028	3	-.002	3	-2.793e-4	3	NC	1	NC	1
199		5	max	.003	1	.032	2	0	1	-1.172e-7	10	NC	1	NC	1
200			min	0	15	-.027	3	-.002	3	-2.793e-4	3	NC	1	NC	1
201		6	max	.002	1	.03	2	0	1	-1.172e-7	10	NC	1	NC	1
202			min	0	15	-.025	3	-.001	3	-2.793e-4	3	NC	1	NC	1
203		7	max	.002	1	.027	2	0	1	-1.172e-7	10	NC	1	NC	1
204			min	0	15	-.023	3	-.001	3	-2.793e-4	3	NC	1	NC	1
205		8	max	.002	1	.025	2	0	1	-1.172e-7	10	NC	1	NC	1
206			min	0	15	-.021	3	-.001	3	-2.793e-4	3	NC	1	NC	1
207		9	max	.002	1	.023	2	0	1	-1.172e-7	10	NC	1	NC	1
208			min	0	15	-.019	3	0	3	-2.793e-4	3	NC	1	NC	1
209		10	max	.002	1	.021	2	0	1	-1.172e-7	10	NC	1	NC	1
210			min	0	15	-.017	3	0	3	-2.793e-4	3	NC	1	NC	1
211		11	max	.002	1	.018	2	0	1	-1.172e-7	10	NC	1	NC	1
212			min	0	15	-.015	3	0	3	-2.793e-4	3	NC	1	NC	1
213		12	max	.001	1	.016	2	0	1	-1.172e-7	10	NC	1	NC	1
214			min	0	15	-.013	3	0	3	-2.793e-4	3	NC	1	NC	1
215		13	max	.001	1	.014	2	0	1	-1.172e-7	10	NC	1	NC	1
216			min	0	15	-.011	3	0	3	-2.793e-4	3	NC	1	NC	1
217		14	max	0	1	.011	2	0	1	-1.172e-7	10	NC	1	NC	1
218			min	0	15	-.009	3	0	3	-2.793e-4	3	NC	1	NC	1
219		15	max	0	1	.009	2	0	1	-1.172e-7	10	NC	1	NC	1
220			min	0	15	-.008	3	0	3	-2.793e-4	3	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-1.172e-7	10	NC	1	NC	1
222			min	0	15	-.006	3	0	3	-2.793e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.005	2	0	1	-1.172e-7	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.793e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.172e-7	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.793e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.172e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.793e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.011	2	0	12	4.126e-4	1	NC	3	NC	1
230			min	-.003	3	-.011	3	-.002	1	-5.975e-4	3	3922.025	2	NC	1
231		2	max	.002	2	.01	2	0	3	3.917e-4	1	NC	3	NC	1
232			min	-.003	3	-.011	3	-.002	1	-5.761e-4	3	4289.148	2	NC	1
233		3	max	.002	2	.009	2	0	3	3.708e-4	1	NC	3	NC	1
234			min	-.003	3	-.01	3	-.002	1	-5.547e-4	3	4727.342	2	NC	1
235		4	max	.002	2	.008	2	0	3	3.5e-4	1	NC	1	NC	1
236			min	-.003	3	-.01	3	-.001	1	-5.333e-4	3	5253.953	2	NC	1
237		5	max	.002	2	.007	2	0	3	3.291e-4	1	NC	1	NC	1
238			min	-.002	3	-.009	3	-.001	1	-5.119e-4	3	5892.148	2	NC	1
239		6	max	.002	2	.006	2	0	3	3.082e-4	1	NC	1	NC	1
240			min	-.002	3	-.009	3	-.001	1	-4.905e-4	3	6673.37	2	NC	1
241		7	max	.001	2	.006	2	0	3	2.873e-4	1	NC	1	NC	1
242			min	-.002	3	-.008	3	-.001	1	-4.691e-4	3	7641.103	2	NC	1
243		8	max	.001	2	.005	2	0	3	2.665e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	-.001	1	-4.477e-4	3	8856.808	2	NC	1
245		9	max	.001	2	.004	2	0	3	2.456e-4	1	NC	1	NC	1
246			min	-.002	3	-.007	3	0	1	-4.263e-4	3	NC	1	NC	1
247		10	max	.001	2	.003	2	0	3	2.247e-4	1	NC	1	NC	1
248			min	-.002	3	-.007	3	0	1	-4.049e-4	3	NC	1	NC	1
249		11	max	0	2	.003	2	0	3	2.038e-4	1	NC	1	NC	1
250			min	-.001	3	-.006	3	0	1	-3.835e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	1.83e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-3.621e-4	3	NC	1	NC	1
253		13	max	0	2	.002	2	0	3	1.621e-4	1	NC	1	NC	1
254			min	-.001	3	-.005	3	0	1	-3.407e-4	3	NC	1	NC	1
255		14	max	0	2	.001	2	0	3	1.412e-4	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-3.193e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	1.203e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.979e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	9.947e-5	1	NC	1	NC	1
260			min	0	3	-.003	3	0	1	-2.766e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	7.859e-5	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-2.552e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	5.772e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.338e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.684e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.124e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.017e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.789e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.606e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-3.148e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.046e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-4.507e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	2	2.486e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-5.865e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	-7.485e-7	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-7.224e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-3.642e-6	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-8.582e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-4.155e-6	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-9.941e-5	1	NC	1	NC	1
281		8	max	0	3	.001	2	0	10	-4.668e-6	10	NC	1	NC	1
282			min	-.001	2	-.006	3	-.002	3	-1.13e-4	1	NC	1	NC	1
283		9	max	.001	3	.001	2	0	10	-5.181e-6	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	3	-1.266e-4	1	NC	1	NC	1
285		10	max	.001	3	.002	2	0	10	-5.694e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.402e-4	1	NC	1	NC	1
287		11	max	.001	3	.002	2	0	10	-6.207e-6	10	NC	1	NC	1
288			min	-.001	2	-.008	3	-.003	3	-1.544e-4	3	NC	1	NC	1
289		12	max	.002	3	.003	2	0	10	-6.72e-6	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	3	-1.8e-4	3	NC	1	NC	1
291		13	max	.002	3	.004	2	0	10	-7.233e-6	10	NC	1	NC	1
292			min	-.002	2	-.009	3	-.003	3	-2.056e-4	3	NC	1	NC	1
293		14	max	.002	3	.005	2	0	10	-7.745e-6	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.003	3	-2.312e-4	3	NC	1	NC	1
295		15	max	.002	3	.005	2	0	10	-8.258e-6	10	NC	1	NC	1
296			min	-.002	2	-.009	3	-.003	1	-2.568e-4	3	8453.801	2	NC	1
297		16	max	.002	3	.006	2	0	10	-8.771e-6	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	1	-2.824e-4	3	7182.902	2	NC	1
299		17	max	.002	3	.007	2	0	10	-9.284e-6	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.004	1	-3.08e-4	3	6195.932	2	NC	1
301		18	max	.002	3	.008	2	0	10	-9.797e-6	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.004	1	-3.336e-4	3	5421.015	2	NC	1
303		19	max	.003	3	.01	2	0	10	-1.031e-5	10	NC	3	NC	1
304			min	-.003	2	-.009	3	-.005	1	-3.592e-4	3	4807.342	2	NC	1
305	M12	1	max	.001	1	.013	2	.004	1	4.03e-4	3	NC	1	NC	2
306			min	0	15	-.011	3	0	10	1.196e-5	10	NC	1	5077.535	1
307		2	max	.001	1	.012	2	.003	1	4.03e-4	3	NC	1	NC	2
308			min	0	15	-.011	3	0	10	1.196e-5	10	NC	1	5537.757	1
309		3	max	.001	1	.011	2	.003	1	4.03e-4	3	NC	1	NC	2
310			min	0	15	-.01	3	0	10	1.196e-5	10	NC	1	6085.561	1
311		4	max	.001	1	.01	2	.003	1	4.03e-4	3	NC	1	NC	2
312			min	0	15	-.009	3	0	10	1.196e-5	10	NC	1	6744.034	1
313		5	max	0	1	.01	2	.003	1	4.03e-4	3	NC	1	NC	2
314			min	0	15	-.009	3	0	10	1.196e-5	10	NC	1	7544.629	1
315		6	max	0	1	.009	2	.002	1	4.03e-4	3	NC	1	NC	2
316			min	0	15	-.008	3	0	10	1.196e-5	10	NC	1	8531.096	1
317		7	max	0	1	.008	2	.002	1	4.03e-4	3	NC	1	NC	2
318			min	0	15	-.007	3	0	10	1.196e-5	10	NC	1	9765.75	1
319		8	max	0	1	.008	2	.002	1	4.03e-4	3	NC	1	NC	1
320			min	0	15	-.007	3	0	10	1.196e-5	10	NC	1	NC	1
321		9	max	0	1	.007	2	.001	1	4.03e-4	3	NC	1	NC	1
322			min	0	15	-.006	3	0	10	1.196e-5	10	NC	1	NC	1
323		10	max	0	1	.006	2	.001	1	4.03e-4	3	NC	1	NC	1
324			min	0	15	-.006	3	0	10	1.196e-5	10	NC	1	NC	1
325		11	max	0	1	.006	2	0	1	4.03e-4	3	NC	1	NC	1
326			min	0	15	-.005	3	0	10	1.196e-5	10	NC	1	NC	1
327		12	max	0	1	.005	2	0	1	4.03e-4	3	NC	1	NC	1
328			min	0	15	-.004	3	0	10	1.196e-5	10	NC	1	NC	1
329		13	max	0	1	.004	2	0	1	4.03e-4	3	NC	1	NC	1
330			min	0	15	-.004	3	0	10	1.196e-5	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	4.03e-4	3	NC	1	NC	1
332			min	0	15	-.003	3	0	10	1.196e-5	10	NC	1	NC	1
333		15	max	0	1	.003	2	0	1	4.03e-4	3	NC	1	NC	1
334			min	0	15	-.002	3	0	10	1.196e-5	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	4.03e-4	3	NC	1	NC	1
336			min	0	15	-.002	3	0	10	1.196e-5	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	4.03e-4	3	NC	1	NC	1
338			min	0	15	-.001	3	0	10	1.196e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.03e-4	3	NC	1	NC	1
340			min	0	15	0	3	0	10	1.196e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.03e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	1.196e-5	10	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.004	3	7.186e-3	2	NC	1	NC	1
344			min	-.01	2	-.023	2	-.001	9	-1.072e-2	3	NC	1	NC	1
345		2	max	.01	3	.017	3	.003	3	3.532e-3	2	NC	4	NC	1
346			min	-.01	2	-.014	2	-.003	1	-5.296e-3	3	5208.248	2	NC	1
347		3	max	.01	3	.007	3	.002	3	2.934e-5	3	NC	4	NC	1
348			min	-.01	2	-.005	2	-.004	1	-2.39e-4	1	2671.152	2	NC	1
349		4	max	.01	3	.003	2	.002	3	3.216e-5	3	NC	4	NC	1
350			min	-.01	2	-.002	3	-.005	1	-2.05e-4	1	1865.479	2	NC	1
351		5	max	.01	3	.009	2	.002	3	3.498e-5	3	NC	4	NC	1
352			min	-.01	2	-.009	3	-.005	1	-1.711e-4	1	1458.549	3	NC	1
353		6	max	.01	3	.015	2	.001	3	3.78e-5	3	NC	4	NC	1
354			min	-.01	2	-.014	3	-.005	1	-1.371e-4	1	1238.862	3	NC	1
355		7	max	.01	3	.019	2	.001	3	4.062e-5	3	NC	4	NC	1
356			min	-.01	2	-.019	3	-.004	1	-1.031e-4	1	1110.418	3	NC	1
357		8	max	.01	3	.023	2	.001	3	4.344e-5	3	NC	4	NC	1
358			min	-.01	2	-.022	3	-.003	1	-6.915e-5	1	1033.535	2	NC	1
359		9	max	.01	3	.025	2	.001	3	4.626e-5	3	NC	4	NC	1
360			min	-.01	2	-.024	3	-.002	1	-3.664e-5	9	985.176	2	NC	1
361		10	max	.009	3	.026	2	.001	3	4.908e-5	3	NC	4	NC	1
362			min	-.01	2	-.024	3	-.001	9	-1.2e-5	9	965.508	2	NC	1
363		11	max	.009	3	.026	2	.001	3	5.19e-5	3	NC	4	NC	1
364			min	-.01	2	-.023	3	0	9	1.082e-6	15	972.541	2	NC	1
365		12	max	.009	3	.024	2	.001	3	6.675e-5	1	NC	4	NC	1
366			min	-.01	2	-.021	3	0	15	2.767e-6	15	1008.859	2	NC	1
367		13	max	.009	3	.021	2	.002	1	1.007e-4	1	NC	4	NC	1
368			min	-.01	2	-.018	3	0	15	4.451e-6	15	1083.063	2	NC	1
369		14	max	.009	3	.016	2	.003	1	1.347e-4	1	NC	4	NC	1
370			min	-.01	2	-.014	3	0	15	6.136e-6	15	1214.89	2	NC	1
371		15	max	.009	3	.01	2	.003	1	1.687e-4	1	NC	4	NC	1
372			min	-.01	2	-.008	3	0	15	7.821e-6	15	1450.816	2	NC	1
373		16	max	.009	3	.002	2	.003	1	1.92e-4	1	NC	4	NC	1
374			min	-.01	2	-.002	3	0	15	8.97e-6	15	1855.89	3	NC	1
375		17	max	.009	3	.006	3	.002	1	7.022e-5	3	NC	4	NC	1
376			min	-.01	2	-.008	2	0	15	-4.447e-5	9	2693.278	3	NC	1
377		18	max	.009	3	.015	3	.001	3	5.354e-3	2	NC	1	NC	1
378			min	-.01	2	-.019	2	0	15	-2.852e-3	3	5281.559	3	NC	1
379		19	max	.009	3	.023	3	0	3	1.08e-2	2	NC	1	NC	1
380			min	-.01	2	-.031	2	0	1	-5.83e-3	3	5656.836	2	NC	1
381	M5	1	max	.03	3	.086	3	.004	3	7.989e-6	3	NC	1	NC	1
382			min	-.032	2	-.072	2	-.002	9	0	15	3674.07	3	NC	1
383		2	max	.03	3	.052	3	.005	3	1.515e-4	3	NC	4	NC	1
384			min	-.032	2	-.044	2	-.001	9	-2.336e-5	9	1621.013	2	NC	1
385		3	max	.03	3	.021	3	.007	3	2.921e-4	3	NC	5	NC	1
386			min	-.032	2	-.016	2	-.001	9	-4.642e-5	9	831.05	2	NC	1
387		4	max	.029	3	.008	2	.008	3	2.804e-4	3	NC	5	NC	1
388			min	-.032	2	-.006	3	-.001	9	-4.439e-5	9	580.067	2	NC	1
389		5	max	.029	3	.03	2	.008	3	2.687e-4	3	NC	5	NC	1
390			min	-.032	2	-.028	3	-.001	9	-4.237e-5	9	458.701	2	8870.913	3
391		6	max	.029	3	.048	2	.009	3	2.57e-4	3	NC	5	NC	1
392			min	-.032	2	-.046	3	-.001	9	-4.034e-5	9	389.529	2	8000.338	3
393		7	max	.029	3	.063	2	.009	3	2.452e-4	3	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394		min	-.032	2	-.059	3	-.001	9	-3.832e-5	9	347.101	2	7595.849	3
395	8	max	.029	3	.074	2	.009	3	2.335e-4	3	NC	5	NC	1
396		min	-.032	2	-.069	3	-.001	9	-3.629e-5	9	320.815	2	7500.76	3
397	9	max	.029	3	.081	2	.009	3	2.218e-4	3	NC	5	NC	1
398		min	-.032	2	-.074	3	-.001	9	-3.427e-5	9	305.721	2	7655.493	3
399	10	max	.029	3	.084	2	.008	3	2.101e-4	3	NC	5	NC	1
400		min	-.032	2	-.076	3	-.001	9	-3.225e-5	9	299.559	2	8052.157	3
401	11	max	.029	3	.083	2	.008	3	1.983e-4	3	NC	5	NC	1
402		min	-.032	2	-.074	3	-.001	9	-3.022e-5	9	301.707	2	8723.047	3
403	12	max	.029	3	.077	2	.007	3	1.866e-4	3	NC	5	NC	1
404		min	-.032	2	-.067	3	0	9	-2.82e-5	9	312.971	2	9747.142	3
405	13	max	.029	3	.067	2	.006	3	1.749e-4	3	NC	5	NC	1
406		min	-.032	2	-.057	3	0	9	-2.617e-5	9	336.029	2	NC	1
407	14	max	.028	3	.052	2	.005	3	1.632e-4	3	NC	5	NC	1
408		min	-.032	2	-.044	3	0	9	-2.415e-5	9	377.042	2	NC	1
409	15	max	.028	3	.032	2	.004	3	1.514e-4	3	NC	5	NC	1
410		min	-.032	2	-.026	3	0	9	-2.212e-5	9	450.535	2	NC	1
411	16	max	.028	3	.006	2	.003	3	1.343e-4	3	NC	5	NC	1
412		min	-.032	2	-.006	3	0	9	-2.182e-5	9	595.279	3	NC	1
413	17	max	.028	3	.019	3	.002	3	-8.186e-8	10	NC	5	NC	1
414		min	-.032	2	-.025	2	0	1	-6.327e-5	1	863.565	3	NC	1
415	18	max	.028	3	.045	3	.002	3	-9.925e-8	10	NC	4	NC	1
416		min	-.032	2	-.061	2	0	1	-3.242e-5	1	1693.439	3	NC	1
417	19	max	.028	3	.073	3	0	3	-3.711e-8	15	NC	3	NC	1
418		min	-.032	2	-.1	2	0	1	-1.428e-6	3	1732.74	2	NC	1
419	M9	1	max	.01	.026	3	.003	3	1.074e-2	3	NC	1	NC	1
420		min	-.01	2	-.023	2	-.002	9	-7.186e-3	2	NC	1	NC	1
421	2	max	.01	3	.016	3	.002	3	5.269e-3	3	NC	4	NC	1
422		min	-.01	2	-.014	2	0	9	-3.532e-3	2	5208.782	2	NC	1
423	3	max	.01	3	.006	3	.002	1	1.368e-4	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	12	-9.755e-5	3	2664.431	3	NC	1
425	4	max	.01	3	.003	2	.002	1	1.071e-4	1	NC	4	NC	1
426		min	-.01	2	-.002	3	-.001	3	-9.916e-5	3	1807.51	3	NC	1
427	5	max	.01	3	.009	2	.003	1	7.736e-5	1	NC	4	NC	1
428		min	-.01	2	-.009	3	-.003	3	-1.008e-4	3	1424.033	3	8736.491	3
429	6	max	.01	3	.015	2	.002	1	4.762e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.004	3	-1.024e-4	3	1215.651	3	7584.921	3
431	7	max	.01	3	.019	2	.002	1	1.788e-5	1	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-1.04e-4	3	1093.148	3	6916.122	3
433	8	max	.01	3	.023	2	0	1	8.374e-6	2	NC	4	NC	1
434		min	-.01	2	-.022	3	-.005	3	-1.056e-4	3	1021.27	3	6540.8	3
435	9	max	.01	3	.025	2	0	2	-7.574e-7	10	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-1.072e-4	3	984.102	3	6373.184	3
437	10	max	.01	3	.026	2	0	2	-2.021e-6	10	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-1.088e-4	3	965.604	2	6376.37	3
439	11	max	.009	3	.026	2	0	10	-3.284e-6	10	NC	4	NC	1
440		min	-.01	2	-.024	3	-.005	3	-1.104e-4	3	972.621	2	6543.311	3
441	12	max	.009	3	.024	2	0	10	-4.547e-6	10	NC	4	NC	1
442		min	-.01	2	-.022	3	-.005	3	-1.308e-4	1	1008.916	2	6892.734	3
443	13	max	.009	3	.021	2	0	10	-5.81e-6	10	NC	4	NC	1
444		min	-.01	2	-.018	3	-.005	3	-1.606e-4	1	1083.081	2	7475.906	3
445	14	max	.009	3	.016	2	0	10	-7.073e-6	10	NC	4	NC	1
446		min	-.01	2	-.014	3	-.004	1	-1.903e-4	1	1214.832	2	8399.652	3
447	15	max	.009	3	.01	2	0	10	-8.336e-6	10	NC	4	NC	1
448		min	-.01	2	-.009	3	-.005	1	-2.201e-4	1	1450.587	2	9888.72	3
449	16	max	.009	3	.002	2	0	10	-9.257e-6	10	NC	4	NC	1
450		min	-.01	2	-.002	3	-.005	1	-2.424e-4	1	1843.18	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.009	3	.006	3	0	10	1.271e-4	3	NC	4	NC	1
452			min	-.01	2	-.008	2	-.004	1	-8.957e-5	1	2675.462	3	NC	1
453		18	max	.009	3	.015	3	0	10	2.953e-3	3	NC	1	NC	1
454			min	-.01	2	-.019	2	-.003	1	-5.354e-3	2	5247.311	3	NC	1
455		19	max	.009	3	.024	3	0	3	5.827e-3	3	NC	1	NC	1
456			min	-.01	2	-.031	2	0	9	-1.08e-2	2	5673.149	2	NC	1
457	M13	1	max	.002	9	.026	3	.01	3	4.002e-3	3	NC	1	NC	1
458			min	-.003	3	-.023	2	-.01	2	-3.419e-3	2	NC	1	NC	1
459		2	max	.002	9	.089	3	.008	3	4.973e-3	3	NC	4	NC	1
460			min	-.003	3	-.064	2	-.007	2	-4.259e-3	2	1733.98	3	NC	1
461		3	max	.002	9	.141	3	.013	1	5.944e-3	3	NC	4	NC	2
462			min	-.004	3	-.1	2	-.006	10	-5.1e-3	2	944.531	3	6025.952	1
463		4	max	.002	9	.176	3	.021	1	6.915e-3	3	NC	5	NC	2
464			min	-.004	3	-.124	2	-.006	10	-5.94e-3	2	723.749	3	4271.749	1
465		5	max	.002	9	.19	3	.023	1	7.886e-3	3	NC	5	NC	2
466			min	-.004	3	-.135	2	-.008	10	-6.781e-3	2	660.352	3	3912.62	1
467		6	max	.002	9	.184	3	.02	9	8.857e-3	3	NC	5	NC	2
468			min	-.004	3	-.132	2	-.01	2	-7.621e-3	2	685.634	3	4516.094	1
469		7	max	.002	9	.162	3	.019	3	9.828e-3	3	NC	5	NC	2
470			min	-.004	3	-.119	2	-.016	2	-8.462e-3	2	800.581	3	7213.187	9
471		8	max	.002	9	.13	3	.023	3	1.08e-2	3	NC	4	NC	1
472			min	-.004	3	-.1	2	-.023	2	-9.302e-3	2	1048.074	3	7988.595	2
473		9	max	.002	9	.1	3	.026	3	1.177e-2	3	NC	4	NC	1
474			min	-.004	3	-.081	2	-.029	2	-1.014e-2	2	1482.075	3	5500.238	2
475		10	max	.002	9	.086	3	.03	3	1.274e-2	3	NC	4	NC	4
476			min	-.004	3	-.072	2	-.032	2	-1.098e-2	2	1834.541	3	4833.182	2
477		11	max	.002	9	.1	3	.032	3	1.177e-2	3	NC	4	NC	1
478			min	-.004	3	-.081	2	-.029	2	-1.014e-2	2	1482.073	3	4873.598	3
479		12	max	.001	9	.13	3	.033	3	1.08e-2	3	NC	4	NC	1
480			min	-.004	3	-.1	2	-.023	2	-9.302e-3	2	1048.072	3	4735.454	3
481		13	max	.001	9	.162	3	.032	3	9.836e-3	3	NC	5	NC	2
482			min	-.004	3	-.119	2	-.016	2	-8.462e-3	2	800.58	3	4945.42	3
483		14	max	.001	9	.185	3	.029	3	8.868e-3	3	NC	5	NC	2
484			min	-.004	3	-.132	2	-.01	2	-7.621e-3	2	685.634	3	4512.772	1
485		15	max	.001	9	.191	3	.026	3	7.899e-3	3	NC	5	NC	2
486			min	-.004	3	-.135	2	-.008	10	-6.781e-3	2	660.351	3	3917.226	1
487		16	max	.001	9	.176	3	.021	3	6.931e-3	3	NC	5	NC	2
488			min	-.004	3	-.124	2	-.006	10	-5.94e-3	2	723.748	3	4284.334	1
489		17	max	.001	9	.142	3	.017	3	5.962e-3	3	NC	4	NC	2
490			min	-.004	3	-.1	2	-.006	10	-5.1e-3	2	944.53	3	6057.316	1
491		18	max	.001	9	.09	3	.013	3	4.994e-3	3	NC	4	NC	1
492			min	-.004	3	-.064	2	-.008	2	-4.259e-3	2	1733.978	3	NC	1
493		19	max	.001	9	.027	3	.01	3	4.026e-3	3	NC	1	NC	1
494			min	-.004	3	-.023	2	-.01	2	-3.419e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.024	3	.009	3	4.483e-3	2	NC	1	NC	1
496			min	0	3	-.031	2	-.01	2	-3.402e-3	3	NC	1	NC	1
497		2	max	0	9	.058	3	.013	3	5.592e-3	2	NC	4	NC	1
498			min	0	3	-.094	2	-.007	2	-4.195e-3	3	1721.106	2	NC	1
499		3	max	0	9	.086	3	.017	3	6.7e-3	2	NC	4	NC	2
500			min	0	3	-.146	2	-.005	10	-4.987e-3	3	935.424	2	6033.072	1
501		4	max	0	9	.107	3	.021	3	7.809e-3	2	NC	5	NC	2
502			min	0	3	-.182	2	-.006	10	-5.78e-3	3	713.939	2	4276.321	1
503		5	max	0	9	.117	3	.024	3	8.917e-3	2	NC	5	NC	2
504			min	0	3	-.198	2	-.007	10	-6.573e-3	3	647.264	2	3917.029	1
505		6	max	0	9	.116	3	.027	3	1.003e-2	2	NC	5	NC	2
506			min	0	3	-.193	2	-.01	2	-7.366e-3	3	665.226	2	4522.571	1
507		7	max	0	9	.107	3	.029	3	1.113e-2	2	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.172	2	-.016	2	-8.159e-3	3	763.794	2	5503.783	3
509	8	max	0	9	.093	3	.03	3	1.224e-2	2	NC	4	NC	1
510		min	0	3	-.142	2	-.023	2	-8.951e-3	3	971.757	2	5288.388	3
511	9	max	0	1	.08	3	.029	3	1.335e-2	2	NC	4	NC	1
512		min	0	3	-.113	2	-.029	2	-9.744e-3	3	1313.852	2	5356.303	3
513	10	max	0	1	.073	3	.028	3	1.446e-2	2	NC	4	NC	4
514		min	0	3	-.1	2	-.032	2	-1.054e-2	3	1571.773	2	4873.24	2
515	11	max	0	1	.08	3	.027	3	1.335e-2	2	NC	4	NC	1
516		min	0	3	-.113	2	-.029	2	-9.742e-3	3	1313.852	2	5550.442	2
517	12	max	0	1	.093	3	.025	3	1.224e-2	2	NC	4	NC	1
518		min	0	3	-.142	2	-.023	2	-8.946e-3	3	971.757	2	6847.591	3
519	13	max	0	1	.107	3	.023	3	1.113e-2	2	NC	5	NC	2
520		min	0	3	-.172	2	-.016	2	-8.151e-3	3	763.794	2	7243.388	9
521	14	max	0	1	.116	3	.021	3	1.003e-2	2	NC	5	NC	2
522		min	0	3	-.193	2	-.01	2	-7.356e-3	3	665.226	2	4528.76	1
523	15	max	0	1	.116	3	.023	1	8.918e-3	2	NC	5	NC	2
524		min	0	3	-.198	2	-.007	10	-6.561e-3	3	647.264	2	3928.8	1
525	16	max	0	1	.107	3	.02	1	7.81e-3	2	NC	5	NC	2
526		min	0	3	-.182	2	-.006	10	-5.765e-3	3	713.939	2	4296.621	1
527	17	max	0	1	.086	3	.013	3	6.702e-3	2	NC	4	NC	2
528		min	0	3	-.146	2	-.005	10	-4.97e-3	3	935.424	2	6076.449	1
529	18	max	0	1	.057	3	.011	3	5.593e-3	2	NC	4	NC	1
530		min	0	3	-.094	2	-.007	2	-4.175e-3	3	1721.106	2	NC	1
531	19	max	0	1	.023	3	.009	3	4.485e-3	2	NC	1	NC	1
532		min	0	3	-.031	2	-.01	2	-3.38e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	4.26e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.425e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	8.674e-4	3	NC	1	NC	1
536		min	0	2	-.004	4	0	3	-4.843e-4	2	NC	1	NC	1
537	3	max	0	3	-.002	15	.003	2	1.309e-3	3	NC	1	NC	1
538		min	0	2	-.008	4	-.004	3	-9.143e-4	2	8289.65	4	8897.116	3
539	4	max	0	3	-.003	15	.006	2	1.75e-3	3	NC	5	NC	4
540		min	0	2	-.012	4	-.008	3	-1.344e-3	2	5687.184	4	4951.606	3
541	5	max	0	3	-.004	15	.01	2	2.192e-3	3	NC	5	NC	4
542		min	0	2	-.016	4	-.013	3	-1.774e-3	2	4437.767	4	3268.51	3
543	6	max	0	3	-.004	15	.015	2	2.633e-3	3	NC	5	NC	4
544		min	-.001	2	-.019	4	-.018	3	-2.204e-3	2	3734.85	4	2389.325	3
545	7	max	0	3	-.005	15	.02	2	3.074e-3	3	NC	15	NC	4
546		min	-.001	2	-.021	4	-.024	3	-2.634e-3	2	3312.138	4	1873.23	3
547	8	max	0	3	-.005	15	.024	2	3.516e-3	3	NC	15	NC	4
548		min	-.002	2	-.023	4	-.03	3	-3.064e-3	2	3058.448	4	1547.932	3
549	9	max	0	3	-.006	15	.028	2	3.957e-3	3	NC	15	NC	4
550		min	-.002	2	-.024	4	-.035	3	-3.494e-3	2	2921.896	4	1334.697	3
551	10	max	.001	3	-.006	15	.031	2	4.399e-3	3	NC	15	NC	4
552		min	-.002	2	-.024	4	-.039	3	-3.924e-3	2	2878.698	4	1193.866	3
553	11	max	.001	3	-.006	15	.033	2	4.84e-3	3	NC	15	NC	4
554		min	-.002	2	-.024	4	-.042	3	-4.354e-3	2	2921.896	4	1104.588	3
555	12	max	.001	3	-.005	15	.033	2	5.281e-3	3	NC	15	NC	4
556		min	-.003	2	-.023	4	-.042	3	-4.784e-3	2	3058.448	4	1056.547	3
557	13	max	.001	3	-.005	15	.032	2	5.723e-3	3	NC	15	NC	4
558		min	-.003	2	-.021	4	-.041	3	-5.214e-3	2	3312.138	4	1046.839	3
559	14	max	.001	3	-.004	15	.028	2	6.164e-3	3	NC	5	NC	4
560		min	-.003	2	-.019	4	-.037	3	-5.644e-3	2	3734.85	4	1080.156	3
561	15	max	.002	3	-.004	15	.022	2	6.606e-3	3	NC	5	NC	4
562		min	-.003	2	-.016	4	-.03	3	-6.075e-3	2	4437.767	4	1173.387	3
563	16	max	.002	3	-.002	2	.015	1	7.047e-3	3	NC	5	NC	4
564		min	-.003	2	-.013	4	-.019	3	-6.505e-3	2	5687.184	4	1372.253	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.002	3	.002	2	.005	1	7.488e-3	3	NC	1	NC	4
566		min	-.004	2	-.009	4	-.005	3	-6.935e-3	2	8289.65	4	1820.09	3
567	18	max	.002	3	.005	2	.014	3	7.93e-3	3	NC	1	NC	4
568		min	-.004	2	-.005	4	-.015	2	-7.365e-3	2	NC	1	3241.861	3
569	19	max	.002	3	.009	2	.037	3	8.371e-3	3	NC	1	NC	1
570		min	-.004	2	-.001	9	-.035	2	-7.795e-3	2	NC	1	NC	1
571	M16A	1	max	0	.002	2	.011	3	2.365e-3	3	NC	1	NC	1
572		min	-.002	3	-.002	3	-.011	2	-2.347e-3	2	NC	1	NC	1
573	2	max	0	2	-.001	15	.003	3	2.278e-3	3	NC	1	NC	1
574		min	-.002	3	-.005	4	-.005	2	-2.243e-3	2	NC	1	9123.984	3
575	3	max	0	2	-.002	15	.004	1	2.191e-3	3	NC	1	NC	4
576		min	-.002	3	-.009	4	-.003	3	-2.138e-3	2	8289.65	4	5170.738	3
577	4	max	0	2	-.003	15	.007	1	2.104e-3	3	NC	5	NC	4
578		min	-.002	3	-.013	4	-.008	3	-2.033e-3	2	5687.184	4	3940.028	3
579	5	max	0	2	-.004	15	.009	1	2.017e-3	3	NC	5	NC	4
580		min	-.002	3	-.016	4	-.011	3	-1.928e-3	2	4437.767	4	3410.058	3
581	6	max	0	2	-.004	15	.011	1	1.929e-3	3	NC	5	NC	4
582		min	-.001	3	-.019	4	-.013	3	-1.823e-3	2	3734.85	4	3183.176	3
583	7	max	0	2	-.005	15	.011	1	1.842e-3	3	NC	15	NC	4
584		min	-.001	3	-.021	4	-.014	3	-1.719e-3	2	3312.138	4	3135.475	3
585	8	max	0	2	-.005	15	.011	1	1.755e-3	3	NC	15	NC	4
586		min	-.001	3	-.023	4	-.014	3	-1.614e-3	2	3058.448	4	3225.715	3
587	9	max	0	2	-.006	15	.011	1	1.668e-3	3	NC	15	NC	4
588		min	-.001	3	-.024	4	-.014	3	-1.509e-3	2	2921.896	4	3450.578	3
589	10	max	0	2	-.006	15	.01	1	1.581e-3	3	NC	15	NC	4
590		min	-.001	3	-.024	4	-.012	3	-1.404e-3	2	2878.698	4	3835.164	3
591	11	max	0	2	-.006	15	.008	1	1.494e-3	3	NC	15	NC	4
592		min	0	3	-.024	4	-.01	3	-1.299e-3	2	2921.896	4	4439.603	3
593	12	max	0	2	-.005	15	.007	1	1.407e-3	3	NC	15	NC	4
594		min	0	3	-.023	4	-.008	3	-1.195e-3	2	3058.448	4	5384.2	3
595	13	max	0	2	-.005	15	.005	1	1.32e-3	3	NC	15	NC	2
596		min	0	3	-.021	4	-.006	3	-1.09e-3	2	3312.138	4	6914.874	3
597	14	max	0	2	-.004	15	.004	1	1.233e-3	3	NC	5	NC	1
598		min	0	3	-.019	4	-.004	3	-9.85e-4	2	3734.85	4	9585.159	3
599	15	max	0	2	-.004	15	.002	1	1.145e-3	3	NC	5	NC	1
600		min	0	3	-.016	4	-.002	3	-8.802e-4	2	4437.767	4	NC	1
601	16	max	0	2	-.003	15	.001	9	1.058e-3	3	NC	5	NC	1
602		min	0	3	-.012	4	0	3	-7.754e-4	2	5687.184	4	NC	1
603	17	max	0	2	-.002	15	0	4	9.712e-4	3	NC	1	NC	1
604		min	0	3	-.008	4	0	2	-6.706e-4	2	8289.65	4	NC	1
605	18	max	0	2	0	15	0	3	8.841e-4	3	NC	1	NC	1
606		min	0	3	-.004	4	0	2	-5.658e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	7.969e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.61e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

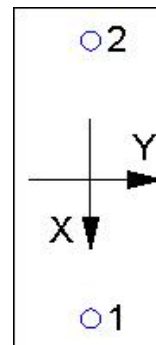
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.