



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	110 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 19.00 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

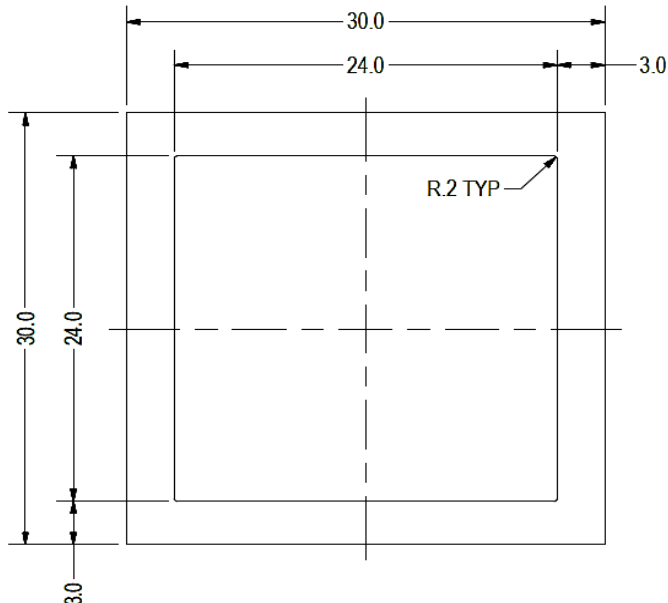
A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.051 k-ft
P_n =	0.238 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	14%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.773 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.852 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	19%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.007 k-ft
P_n =	0.246 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	17%



A cross brace kit is required every 11 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

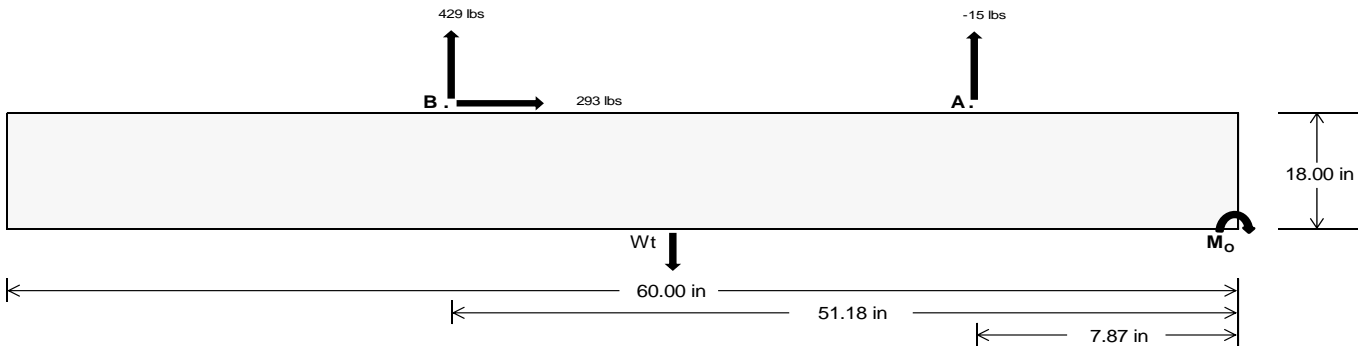
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	19.64	1864.99	k
Compressive Load =	1291.98	1371.60	k
Lateral Load =	41.51	1270.64	k
Moment (Weak Axis) =	0.07	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27117.2$ in-lbs
Resisting Force Required = 903.91 lbs
S.F. = 1.67
Weight Required = 1506.51 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 292.97 lbs
Friction = 0.4
Weight Required = 732.44 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 292.97 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

		Ballast Width			
		21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$		1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	514 lbs	514 lbs	514 lbs	514 lbs	358 lbs	358 lbs	358 lbs	358 lbs	603 lbs	603 lbs	603 lbs	603 lbs	31 lbs	31 lbs	31 lbs	31 lbs
F_B	348 lbs	348 lbs	348 lbs	348 lbs	570 lbs	570 lbs	570 lbs	570 lbs	652 lbs	652 lbs	652 lbs	652 lbs	-858 lbs	-858 lbs	-858 lbs	-858 lbs
F_V	68 lbs	68 lbs	68 lbs	68 lbs	538 lbs	538 lbs	538 lbs	538 lbs	449 lbs	449 lbs	449 lbs	449 lbs	-586 lbs	-586 lbs	-586 lbs	-586 lbs
P_{total}	2765 lbs	2856 lbs	2946 lbs	3037 lbs	2831 lbs	2922 lbs	3013 lbs	3103 lbs	3158 lbs	3249 lbs	3340 lbs	3430 lbs	314 lbs	369 lbs	423 lbs	477 lbs
M	437 lbs-ft	437 lbs-ft	437 lbs-ft	437 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	462 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	633 lbs-ft	693 lbs-ft	693 lbs-ft	693 lbs-ft	693 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.20 ft	1.88 ft	1.64 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	256.1 psf	254.3 psf	252.7 psf	251.3 psf	260.3 psf	258.3 psf	256.6 psf	254.9 psf	274.2 psf	271.6 psf	269.2 psf	267.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	375.9 psf	368.7 psf	362.2 psf	356.1 psf	386.9 psf	379.2 psf	372.1 psf	365.7 psf	447.8 psf	437.3 psf	427.7 psf	419.0 psf	404.5 psf	215.8 psf	170.6 psf	151.7 psf

Maximum Bearing Pressure = 448 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

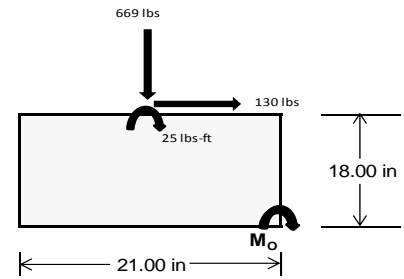
Overturning Check

$M_o = 365.1 \text{ ft-lbs}$
 Resisting Force Required = 417.27 lbs
 S.F. = 1.67
 Weight Required = 695.44 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	150 lbs	147 lbs	87 lbs	310 lbs	669 lbs	261 lbs	89 lbs	-6 lbs	29 lbs
F_v	22 lbs	172 lbs	23 lbs	15 lbs	130 lbs	18 lbs	22 lbs	172 lbs	23 lbs
P_{total}	2506 lbs	2503 lbs	2443 lbs	2553 lbs	2912 lbs	2504 lbs	778 lbs	683 lbs	718 lbs
M	63 lbs-ft	292 lbs-ft	68 lbs-ft	41 lbs-ft	220 lbs-ft	54 lbs-ft	65 lbs-ft	292 lbs-ft	68 lbs-ft
e	0.03 ft	0.12 ft	0.03 ft	0.02 ft	0.08 ft	0.02 ft	0.08 ft	0.43 ft	0.09 ft
$L/6$	0.29 ft	1.52 ft	1.69 ft	1.72 ft	1.60 ft	1.71 ft	1.58 ft	0.90 ft	1.56 ft
f_{min}	261.7 sqft	171.5 sqft	252.5 sqft	275.8 sqft	246.4 sqft	265.2 sqft	63.5 sqft	-36.3 sqft	55.4 sqft
f_{max}	311.1 psf	400.6 psf	305.9 psf	307.8 psf	419.2 psf	307.3 psf	114.2 psf	192.4 psf	108.7 psf



Maximum Bearing Pressure = 419 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.423 k
Allowable Uplift =	1.214 k
Utilization =	<u>35%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.087 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.994 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Diagonal Strut

Maximum Axial Load =	0.773 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.170 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.246 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.108 in
	<u>0.108 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$226.543$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 87.00 \text{ in}$$

$$J = 0.255$$

$$235.251$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.211 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	263.426	1	-.031	15	.499	1	0	12	.001	4	0	15
30			min	-358.668	3	-.129	4	-.4	5	-.001	1	0	3	0	6
31		16	max	263.561	1	-.044	15	.499	1	0	12	.001	4	0	15
32			min	-358.567	3	-.186	4	-.523	5	-.001	1	0	3	0	6
33		17	max	263.696	1	-.058	15	.499	1	0	12	.001	4	0	15
34			min	-358.465	3	-.244	4	-.647	5	-.001	1	0	3	0	6
35		18	max	263.831	1	-.071	15	.499	1	0	12	.001	1	0	15
36			min	-358.364	3	-.301	4	-.77	5	-.001	1	0	3	0	6
37		19	max	263.966	1	-.085	15	.499	1	0	12	.001	1	0	15
38			min	-358.263	3	-.359	4	-.893	5	-.001	1	0	3	0	6
39	M3	1	max	196.256	2	1.733	6	-.037	12	0	5	.002	1	0	6
40			min	-214.096	3	.407	15	-1.419	4	0	1	0	12	0	15
41		2	max	196.186	2	1.557	6	-.037	12	0	5	.002	1	0	2
42			min	-214.149	3	.366	15	-1.285	4	0	1	0	12	0	3
43		3	max	196.116	2	1.381	6	-.037	12	0	5	.002	1	0	2
44			min	-214.201	3	.324	15	-1.152	4	0	1	0	15	0	3
45		4	max	196.046	2	1.204	6	-.037	12	0	5	.002	1	0	15
46			min	-214.254	3	.283	15	-1.018	4	0	1	0	5	0	4
47		5	max	195.976	2	1.028	6	-.037	12	0	5	.001	1	0	15
48			min	-214.306	3	.241	15	-.884	4	0	1	0	5	0	4
49		6	max	195.906	2	.852	6	-.037	12	0	5	.001	1	0	15
50			min	-214.359	3	.2	15	-.75	4	0	1	0	5	0	4
51		7	max	195.836	2	.675	6	-.037	12	0	5	.001	1	0	15
52			min	-214.411	3	.158	15	-.617	4	0	1	0	5	0	4
53		8	max	195.766	2	.499	6	-.037	12	0	5	.001	1	0	15
54			min	-214.464	3	.117	15	-.569	1	0	1	0	5	-.001	4
55		9	max	195.696	2	.322	6	-.037	12	0	5	0	1	0	15
56			min	-214.516	3	.075	15	-.569	1	0	1	0	5	-.001	4
57		10	max	195.626	2	.146	6	-.037	12	0	5	0	1	0	15
58			min	-214.569	3	.034	15	-.569	1	0	1	0	5	-.001	4
59		11	max	195.556	2	.003	2	.048	5	0	5	0	1	0	15
60			min	-214.621	3	-.053	3	-.569	1	0	1	0	5	-.001	4
61		12	max	195.486	2	-.049	15	.181	5	0	5	0	1	0	15
62			min	-214.674	3	-.207	4	-.569	1	0	1	0	5	-.001	4
63		13	max	195.416	2	-.09	15	.315	5	0	5	0	1	0	15
64			min	-214.726	3	-.383	4	-.569	1	0	1	0	5	-.001	4
65		14	max	195.346	2	-.132	15	.449	5	0	5	0	1	0	15
66			min	-214.779	3	-.559	4	-.569	1	0	1	0	5	-.001	4
67		15	max	195.276	2	-.173	15	.582	5	0	5	0	1	0	15
68			min	-214.831	3	-.736	4	-.569	1	0	1	0	5	0	4
69		16	max	195.206	2	-.215	15	.716	5	0	5	0	1	0	15
70			min	-214.884	3	-.912	4	-.569	1	0	1	0	5	0	4
71		17	max	195.136	2	-.256	15	.85	5	0	5	0	10	0	15
72			min	-214.936	3	-1.089	4	-.569	1	0	1	0	5	0	4
73		18	max	195.066	2	-.298	15	.983	5	0	5	0	12	0	15
74			min	-214.989	3	-1.265	4	-.569	1	0	1	0	1	0	4
75		19	max	194.996	2	-.339	15	1.117	5	0	5	0	5	0	1
76			min	-215.041	3	-1.441	4	-.569	1	0	1	0	1	0	1
77	M4	1	max	399.345	1	0	1	-.223	12	0	1	0	5	0	1
78			min	25.321	15	0	1	-31.348	4	0	1	0	2	0	1
79		2	max	399.41	1	0	1	-.223	12	0	1	0	12	0	1
80			min	25.34	15	0	1	-31.404	4	0	1	-.003	4	0	1
81		3	max	399.475	1	0	1	-.223	12	0	1	0	12	0	1
82			min	25.36	15	0	1	-31.46	4	0	1	-.006	4	0	1
83		4	max	399.539	1	0	1	-.223	12	0	1	0	12	0	1
84			min	25.379	15	0	1	-31.516	4	0	1	-.008	4	0	1
85		5	max	399.604	1	0	1	-.223	12	0	1	0	12	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	25.399	15	0	1	-31.572	4	0	1	-.011	4	0	1
87		6	max	399.669	1	0	1	-.223	12	0	1	0	12	0	1
88			min	25.419	15	0	1	-31.628	4	0	1	-.014	4	0	1
89		7	max	399.733	1	0	1	-.223	12	0	1	0	12	0	1
90			min	25.438	15	0	1	-31.684	4	0	1	-.017	4	0	1
91		8	max	399.798	1	0	1	-.223	12	0	1	0	12	0	1
92			min	25.458	15	0	1	-31.741	4	0	1	-.02	4	0	1
93		9	max	399.863	1	0	1	-.223	12	0	1	0	12	0	1
94			min	25.477	15	0	1	-31.797	4	0	1	-.023	4	0	1
95		10	max	399.928	1	0	1	-.223	12	0	1	0	12	0	1
96			min	25.497	15	0	1	-31.853	4	0	1	-.025	4	0	1
97		11	max	399.992	1	0	1	-.223	12	0	1	0	12	0	1
98			min	25.516	15	0	1	-31.909	4	0	1	-.028	4	0	1
99		12	max	400.057	1	0	1	-.223	12	0	1	0	12	0	1
100			min	25.536	15	0	1	-31.965	4	0	1	-.031	4	0	1
101		13	max	400.122	1	0	1	-.223	12	0	1	0	12	0	1
102			min	25.555	15	0	1	-32.021	4	0	1	-.034	4	0	1
103		14	max	400.186	1	0	1	-.223	12	0	1	0	12	0	1
104			min	25.575	15	0	1	-32.077	4	0	1	-.037	4	0	1
105		15	max	400.251	1	0	1	-.223	12	0	1	0	12	0	1
106			min	25.594	15	0	1	-32.133	4	0	1	-.04	4	0	1
107		16	max	400.316	1	0	1	-.223	12	0	1	0	12	0	1
108			min	25.614	15	0	1	-32.189	4	0	1	-.043	4	0	1
109		17	max	400.381	1	0	1	-.223	12	0	1	0	12	0	1
110			min	25.633	15	0	1	-32.245	4	0	1	-.045	4	0	1
111		18	max	400.445	1	0	1	-.223	12	0	1	0	12	0	1
112			min	25.653	15	0	1	-32.301	4	0	1	-.048	4	0	1
113		19	max	400.51	1	0	1	-.223	12	0	1	0	12	0	1
114			min	25.672	15	0	1	-32.357	4	0	1	-.051	4	0	1
115	M6	1	max	849.622	1	.665	6	1.309	4	0	3	0	3	0	1
116			min	-1169.541	3	.148	15	-.146	3	0	5	0	11	0	1
117		2	max	849.757	1	.607	6	1.186	4	0	3	0	4	0	15
118			min	-1169.44	3	.134	15	-.146	3	0	5	0	11	0	6
119		3	max	849.892	1	.55	6	1.063	4	0	3	0	4	0	15
120			min	-1169.339	3	.121	15	-.146	3	0	5	0	11	0	6
121		4	max	850.026	1	.492	6	.94	4	0	3	0	4	0	15
122			min	-1169.238	3	.107	15	-.146	3	0	5	0	10	0	6
123		5	max	850.161	1	.447	2	.816	4	0	3	0	4	0	15
124			min	-1169.137	3	.094	15	-.146	3	0	5	0	10	0	6
125		6	max	850.296	1	.402	2	.693	4	0	3	.001	4	0	15
126			min	-1169.036	3	.078	12	-.146	3	0	5	0	10	0	6
127		7	max	850.431	1	.357	2	.57	4	0	3	.001	4	0	15
128			min	-1168.934	3	.055	12	-.146	3	0	5	0	12	0	6
129		8	max	850.566	1	.312	2	.447	4	0	3	.001	4	0	15
130			min	-1168.833	3	.033	12	-.146	3	0	5	0	3	0	2
131		9	max	850.701	1	.268	2	.324	4	0	3	.001	4	0	15
132			min	-1168.732	3	.004	3	-.146	3	0	5	0	3	0	2
133		10	max	850.836	1	.223	2	.201	4	0	3	.001	4	0	15
134			min	-1168.631	3	-.03	3	-.146	3	0	5	0	3	0	2
135		11	max	850.97	1	.178	2	.137	1	0	3	.001	4	0	12
136			min	-1168.53	3	-.064	3	-.146	3	0	5	0	3	0	2
137		12	max	851.105	1	.133	2	.137	1	0	3	.001	4	0	12
138			min	-1168.429	3	-.097	3	-.146	3	0	5	0	3	0	2
139		13	max	851.24	1	.089	2	.137	1	0	3	.001	4	0	12
140			min	-1168.328	3	-.131	3	-.217	5	0	5	0	3	0	2
141		14	max	851.375	1	.044	2	.137	1	0	3	.001	4	0	12
142			min	-1168.226	3	-.164	3	-.34	5	0	5	0	3	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	851.51	1	-0.001	2	.137	1	0	3	.001	4	0	12
144		min	-1168.125	3	-.198	3	-.463	5	0	5	0	3	0	2
145	16	max	851.645	1	-.046	2	.137	1	0	3	.001	4	0	3
146		min	-1168.024	3	-.232	3	-.586	5	0	5	0	3	0	2
147	17	max	851.78	1	-.068	15	.137	1	0	3	.001	4	0	3
148		min	-1167.923	3	-.265	3	-.71	5	0	5	0	3	0	2
149	18	max	851.915	1	-.082	15	.137	1	0	3	.001	4	0	3
150		min	-1167.822	3	-.313	4	-.833	5	0	5	0	3	0	2
151	19	max	852.049	1	-.095	15	.137	1	0	3	0	4	0	3
152		min	-1167.721	3	-.371	4	-.956	5	0	5	0	3	0	2
153	M7	1	max	772.583	2	1.756	.027	3	0	14	0	4	0	2
154		min	-672.763	3	.42	15	-1.287	5	0	3	0	3	0	3
155	2	max	772.513	2	1.58	4	.027	3	0	14	0	4	0	2
156		min	-672.815	3	.378	15	-1.153	5	0	3	0	3	0	3
157	3	max	772.443	2	1.403	4	.027	3	0	14	0	2	0	2
158		min	-672.868	3	.337	15	-1.02	5	0	3	0	3	0	3
159	4	max	772.373	2	1.227	4	.027	3	0	14	0	2	0	2
160		min	-672.92	3	.295	15	-.886	5	0	3	0	5	0	3
161	5	max	772.303	2	1.051	4	.027	3	0	14	0	2	0	15
162		min	-672.973	3	.254	15	-.752	5	0	3	0	5	0	3
163	6	max	772.233	2	.874	4	.027	3	0	14	0	2	0	15
164		min	-673.025	3	.212	15	-.619	5	0	3	0	5	0	3
165	7	max	772.163	2	.698	4	.027	3	0	14	0	2	0	15
166		min	-673.078	3	.171	15	-.485	5	0	3	0	5	0	6
167	8	max	772.093	2	.521	4	.027	3	0	14	0	2	0	15
168		min	-673.13	3	.123	12	-.351	5	0	3	0	5	-.001	6
169	9	max	772.023	2	.349	2	.027	3	0	14	0	2	0	15
170		min	-673.183	3	.054	12	-.218	5	0	3	0	5	-.001	6
171	10	max	771.953	2	.211	2	.027	3	0	14	0	2	0	15
172		min	-673.235	3	-.033	3	-.084	5	0	3	0	5	-.001	6
173	11	max	771.883	2	.074	2	.052	4	0	14	0	2	0	15
174		min	-673.288	3	-.137	3	-.005	2	0	3	0	5	-.001	6
175	12	max	771.813	2	-.036	15	.185	4	0	14	0	2	0	15
176		min	-673.34	3	-.24	3	-.005	2	0	3	0	5	-.001	6
177	13	max	771.743	2	-.078	15	.319	4	0	14	0	2	0	15
178		min	-673.393	3	-.361	6	-.005	2	0	3	0	5	-.001	6
179	14	max	771.673	2	-.119	15	.453	4	0	14	0	2	0	15
180		min	-673.445	3	-.537	6	-.005	2	0	3	0	5	-.001	6
181	15	max	771.603	2	-.161	15	.586	4	0	14	0	2	0	15
182		min	-673.498	3	-.714	6	-.005	2	0	3	0	5	0	6
183	16	max	771.533	2	-.202	15	.72	4	0	14	0	2	0	15
184		min	-673.55	3	-.89	6	-.005	2	0	3	0	5	0	6
185	17	max	771.463	2	-.244	15	.854	4	0	14	0	2	0	15
186		min	-673.603	3	-1.067	6	-.005	2	0	3	0	5	0	6
187	18	max	771.393	2	-.285	15	.987	4	0	14	0	2	0	15
188		min	-673.655	3	-1.243	6	-.005	2	0	3	0	3	0	6
189	19	max	771.323	2	-.327	15	1.121	4	0	14	0	14	0	1
190		min	-673.708	3	-1.419	6	-.005	2	0	3	0	3	0	1
191	M8	1	max	992.669	1	0	.807	1	0	1	0	4	0	1
192		min	35.361	15	0	1	-31.29	4	0	1	0	1	0	1
193	2	max	992.734	1	0	1	.807	1	0	1	0	1	0	1
194		min	35.381	15	0	1	-31.347	4	0	1	-.003	4	0	1
195	3	max	992.799	1	0	1	.807	1	0	1	0	1	0	1
196		min	35.4	15	0	1	-31.403	4	0	1	-.006	4	0	1
197	4	max	992.863	1	0	1	.807	1	0	1	0	1	0	1
198		min	35.42	15	0	1	-31.459	4	0	1	-.008	4	0	1
199	5	max	992.928	1	0	1	.807	1	0	1	0	1	0	1



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 35° 110mph 30psf 7.25ft 7-10Pa Page 25



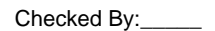
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	276.868	1	-.01	15	.005	3	.001	1	.002	4	0	15
258			min	-331.591	3	-.112	1	-.277	4	-.002	5	0	1	0	4
259		16	max	277.003	1	-.024	15	.005	3	.001	1	.001	5	0	15
260			min	-331.49	3	-.157	1	-.4	4	-.002	5	0	1	0	4
261		17	max	277.138	1	-.037	15	.005	3	.001	1	.001	5	0	15
262			min	-331.389	3	-.214	6	-.523	4	-.002	5	0	1	0	4
263		18	max	277.273	1	-.051	15	.005	3	.001	1	.001	5	0	12
264			min	-331.288	3	-.272	6	-.646	4	-.002	5	0	1	0	4
265		19	max	277.408	1	-.064	15	.005	3	.001	1	.001	5	0	12
266			min	-331.186	3	-.329	6	-.769	4	-.002	5	0	1	0	4
267	M11	1	max	195.97	2	1.721	6	.634	1	.002	4	0	5	0	1
268			min	-214.751	3	.398	15	-1.204	5	0	10	-.002	1	0	15
269		2	max	195.9	2	1.545	6	.634	1	.002	4	0	5	0	1
270			min	-214.804	3	.357	15	-1.07	5	0	10	-.002	1	0	3
271		3	max	195.83	2	1.368	6	.634	1	.002	4	0	5	0	1
272			min	-214.856	3	.315	15	-.937	5	0	10	-.002	1	0	3
273		4	max	195.76	2	1.192	6	.634	1	.002	4	0	5	0	15
274			min	-214.909	3	.274	15	-.803	5	0	10	-.002	1	0	4
275		5	max	195.69	2	1.015	6	.634	1	.002	4	0	3	0	15
276			min	-214.961	3	.232	15	-.669	5	0	10	-.001	1	0	4
277		6	max	195.62	2	.839	6	.634	1	.002	4	0	3	0	15
278			min	-215.014	3	.191	15	-.536	5	0	10	-.001	1	0	4
279		7	max	195.55	2	.663	6	.634	1	.002	4	0	3	0	15
280			min	-215.066	3	.149	15	-.402	5	0	10	-.001	1	-.001	4
281		8	max	195.48	2	.486	6	.634	1	.002	4	0	3	0	15
282			min	-215.119	3	.108	15	-.268	5	0	10	0	1	-.001	4
283		9	max	195.41	2	.31	6	.634	1	.002	4	0	3	0	15
284			min	-215.171	3	.067	15	-.135	5	0	10	0	1	-.001	4
285		10	max	195.34	2	.143	1	.634	1	.002	4	0	3	0	15
286			min	-215.224	3	.025	15	-.009	3	0	10	0	1	-.001	4
287		11	max	195.27	2	.005	1	.634	1	.002	4	0	3	0	15
288			min	-215.276	3	-.07	3	-.009	3	0	10	0	1	-.001	4
289		12	max	195.2	2	-.058	15	.634	1	.002	4	0	3	0	15
290			min	-215.329	3	-.22	4	-.009	3	0	10	0	1	-.001	4
291		13	max	195.13	2	-.099	15	.634	1	.002	4	0	3	0	15
292			min	-215.381	3	-.396	4	-.009	3	0	10	0	1	-.001	4
293		14	max	195.06	2	-.141	15	.676	4	.002	4	0	3	0	15
294			min	-215.434	3	-.572	4	-.009	3	0	10	0	1	-.001	4
295		15	max	194.99	2	-.182	15	.809	4	.002	4	0	3	0	15
296			min	-215.486	3	-.749	4	-.009	3	0	10	0	1	0	4
297		16	max	194.92	2	-.224	15	.943	4	.002	4	0	4	0	15
298			min	-215.539	3	-.925	4	-.009	3	0	10	0	10	0	4
299		17	max	194.85	2	-.265	15	1.077	4	.002	4	0	4	0	15
300			min	-215.591	3	-1.102	4	-.009	3	0	10	0	10	0	4
301		18	max	194.78	2	-.307	15	1.21	4	.002	4	0	4	0	15
302			min	-215.644	3	-1.278	4	-.009	3	0	10	0	10	0	4
303		19	max	194.71	2	-.348	15	1.344	4	.002	4	.001	4	0	1
304			min	-215.696	3	-1.454	4	-.009	3	0	10	0	10	0	1
305	M12	1	max	399.005	1	0	1	3.998	1	0	1	0	4	0	1
306			min	9.131	15	0	1	-28.614	5	0	1	0	3	0	1
307		2	max	399.07	1	0	1	3.998	1	0	1	0	1	0	1
308			min	9.15	15	0	1	-28.67	5	0	1	-.003	5	0	1
309		3	max	399.135	1	0	1	3.998	1	0	1	0	1	0	1
310			min	9.17	15	0	1	-28.726	5	0	1	-.005	5	0	1
311		4	max	399.199	1	0	1	3.998	1	0	1	.001	1	0	1
312			min	9.189	15	0	1	-28.782	5	0	1	-.008	5	0	1
313		5	max	399.264	1	0	1	3.998	1	0	1	.001	1	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	116.477	3	4.659	9	-4.471	12	0	12	-.004	12	.181	2
372			min	-11.955	10	-30.42	2	-79.453	1	0	1	-.086	1	-.119	3
373		16	max	92.173	2	141.363	2	-4.509	12	0	1	-.005	12	.186	2
374			min	2.844	15	-201.461	3	-79.913	1	0	5	-.103	1	-.115	3
375		17	max	92.333	2	141.134	2	-4.509	12	0	1	-.006	12	.156	2
376			min	2.893	15	-201.633	3	-79.913	1	0	5	-.121	1	-.071	3
377		18	max	-6.959	12	360.314	2	-4.732	12	0	3	-.007	12	.078	2
378			min	-150.288	1	-163.007	3	-81.994	1	0	2	-.138	1	-.036	3
379		19	max	-6.879	12	360.085	2	-4.732	12	0	3	-.008	12	0	2
380			min	-150.128	1	-163.179	3	-81.994	1	0	2	-.156	1	0	3
381	M5	1	max	329.335	1	1112.179	3	-.077	10	0	1	.042	4	0	3
382			min	12.051	15	-851.007	1	-47.347	3	0	5	0	10	0	2
383		2	max	329.495	1	1112.007	3	-.077	10	0	1	.036	4	.184	1
384			min	12.099	15	-851.236	1	-47.347	3	0	5	-.005	3	-.241	3
385		3	max	361.278	3	5.731	9	5.466	3	0	3	.03	4	.365	1
386			min	-70.493	2	-103.508	2	-22.673	4	0	4	-.015	3	-.477	3
387		4	max	361.399	3	5.541	9	5.466	3	0	3	.025	4	.375	1
388			min	-70.333	2	-103.736	2	-22.431	4	0	4	-.014	3	-.469	3
389		5	max	361.519	3	5.35	9	5.466	3	0	3	.021	4	.392	2
390			min	-70.172	2	-103.965	2	-22.189	4	0	4	-.013	3	-.462	3
391		6	max	361.639	3	5.159	9	5.466	3	0	3	.016	4	.415	2
392			min	-70.012	2	-104.194	2	-21.947	4	0	4	-.011	3	-.454	3
393		7	max	361.759	3	4.969	9	5.466	3	0	3	.011	4	.438	2
394			min	-69.852	2	-104.422	2	-21.705	4	0	4	-.01	3	-.446	3
395		8	max	361.879	3	4.778	9	5.466	3	0	3	.006	4	.46	2
396			min	-69.692	2	-104.651	2	-21.463	4	0	4	-.009	3	-.439	3
397		9	max	361.999	3	4.588	9	5.466	3	0	3	.002	5	.483	2
398			min	-69.532	2	-104.88	2	-21.221	4	0	4	-.008	3	-.431	3
399		10	max	362.119	3	4.397	9	5.466	3	0	3	0	10	.506	2
400			min	-69.372	2	-105.109	2	-20.979	4	0	4	-.007	3	-.423	3
401		11	max	362.239	3	4.206	9	5.466	3	0	3	0	10	.529	2
402			min	-69.211	2	-105.337	2	-20.737	4	0	4	-.007	4	-.415	3
403		12	max	362.36	3	4.016	9	5.466	3	0	3	0	10	.552	2
404			min	-69.051	2	-105.566	2	-20.495	4	0	4	-.012	4	-.407	3
405		13	max	362.48	3	3.825	9	5.466	3	0	3	0	10	.574	2
406			min	-68.891	2	-105.795	2	-20.253	4	0	4	-.016	4	-.399	3
407		14	max	362.6	3	3.635	9	5.466	3	0	3	0	10	.597	2
408			min	-68.731	2	-106.024	2	-20.011	4	0	4	-.021	4	-.391	3
409		15	max	362.72	3	3.444	9	5.466	3	0	3	0	10	.62	2
410			min	-68.571	2	-106.252	2	-19.769	4	0	4	-.025	4	-.383	3
411		16	max	294.224	2	578.051	2	5.45	3	0	1	0	3	.638	2
412			min	3.897	15	-633.084	3	-18.421	4	0	4	-.029	4	-.37	3
413		17	max	294.384	2	577.823	2	5.45	3	0	1	.001	3	.512	2
414			min	3.946	15	-633.256	3	-18.179	4	0	4	-.033	4	-.233	3
415		18	max	-13.654	12	1187.431	2	4.971	3	0	4	.002	3	.257	2
416			min	-330.111	1	-535.972	3	-48.129	5	0	1	-.043	4	-.116	3
417		19	max	-13.574	12	1187.203	2	4.971	3	0	4	.003	3	0	3
418			min	-329.951	1	-536.143	3	-47.887	5	0	1	-.054	4	0	2
419	M9	1	max	149.915	1	336.368	3	206.686	4	0	3	-.003	15	0	2
420			min	4.872	15	-257.031	1	9.05	10	0	1	-.156	1	0	3
421		2	max	150.076	1	336.197	3	206.928	4	0	3	.035	5	.056	1
422			min	4.92	15	-257.26	1	9.05	10	0	1	-.134	1	-.073	3
423		3	max	115.267	3	6.927	9	75.671	1	0	1	.074	5	.111	1
424			min	-13.008	10	-27.683	2	-28.036	5	0	5	-.112	1	-.145	3
425		4	max	115.387	3	6.736	9	75.671	1	0	1	.068	5	.112	1
426			min	-12.875	10	-27.912	2	-27.794	5	0	5	-.095	1	-.143	3
427		5	max	115.507	3	6.545	9	75.671	1	0	1	.062	5	.118	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-12.741	10	-28.141	2	-27.552	5	0	5	-.079	1	-.141	3
429	6	max	115.627	3	6.355	9	75.671	1	0	1	.056	5	.124	2
430		min	-12.608	10	-28.369	2	-27.31	5	0	5	-.062	1	-.139	3
431	7	max	115.747	3	6.164	9	75.671	1	0	1	.05	5	.13	2
432		min	-12.474	10	-28.598	2	-27.068	5	0	5	-.046	1	-.137	3
433	8	max	115.867	3	5.973	9	75.671	1	0	1	.044	5	.136	2
434		min	-12.341	10	-28.827	2	-26.826	5	0	5	-.03	1	-.135	3
435	9	max	115.988	3	5.783	9	75.671	1	0	1	.038	5	.142	2
436		min	-12.207	10	-29.056	2	-26.584	5	0	5	-.013	1	-.132	3
437	10	max	116.108	3	5.592	9	75.671	1	0	1	.033	4	.149	2
438		min	-12.074	10	-29.284	2	-26.342	5	0	5	0	2	-.13	3
439	11	max	116.228	3	5.402	9	75.671	1	0	1	.031	4	.155	2
440		min	-11.94	10	-29.513	2	-26.1	5	0	5	.002	10	-.128	3
441	12	max	116.348	3	5.211	9	75.671	1	0	1	.036	1	.162	2
442		min	-11.807	10	-29.742	2	-25.858	5	0	5	.004	10	-.126	3
443	13	max	116.468	3	5.02	9	75.671	1	0	1	.052	1	.168	2
444		min	-11.673	10	-29.971	2	-25.616	5	0	5	.006	10	-.124	3
445	14	max	116.588	3	4.83	9	75.671	1	0	1	.069	1	.175	2
446		min	-11.54	10	-30.199	2	-25.374	5	0	5	.006	12	-.121	3
447	15	max	116.708	3	4.639	9	75.671	1	0	1	.085	1	.181	2
448		min	-11.406	10	-30.428	2	-25.132	5	0	5	.003	15	-.119	3
449	16	max	92.546	2	141.103	2	76.188	1	0	10	.103	1	.186	2
450		min	4.435	15	-201.973	3	-23.694	5	0	4	0	15	-.115	3
451	17	max	92.706	2	140.875	2	76.188	1	0	10	.119	1	.156	2
452		min	4.484	15	-202.144	3	-23.452	5	0	4	-.004	5	-.071	3
453	18	max	-.705	15	360.315	2	80.312	1	0	2	.137	1	.078	2
454		min	-149.923	1	-163.003	3	-51.728	5	0	3	-.015	5	-.036	3
455	19	max	-.657	15	360.086	2	80.312	1	0	2	.154	1	0	2
456		min	-149.763	1	-163.175	3	-51.486	5	0	3	-.026	5	0	3
457	M13	1	max	206.707	4	256.608	1	-4.872	15	0	.156	1	0	1
458		min	9.054	10	-336.363	3	-149.895	1	0	3	.003	15	0	3
459	2	max	198.795	4	181.028	1	-3.292	15	0	2	.049	1	.231	3
460		min	9.054	10	-237.217	3	-114.904	1	0	3	0	15	-.176	1
461	3	max	190.883	4	105.448	1	-1.712	15	0	2	.003	3	.382	3
462		min	9.054	10	-138.071	3	-79.913	1	0	3	-.029	1	-.292	1
463	4	max	182.972	4	29.868	1	-.133	15	0	2	-.001	12	.453	3
464		min	9.054	10	-38.925	3	-44.922	1	0	3	-.079	1	-.346	1
465	5	max	175.06	4	60.221	3	2.129	5	0	2	-.002	15	.445	3
466		min	9.054	10	-45.712	1	-9.93	1	0	3	-.102	1	-.34	1
467	6	max	167.148	4	159.367	3	25.061	1	0	2	0	15	.356	3
468		min	9.054	10	-121.293	1	.282	12	0	3	-.095	1	-.272	1
469	7	max	159.236	4	258.513	3	60.052	1	0	2	.004	5	.188	3
470		min	9.054	10	-196.873	1	1.814	12	0	3	-.061	1	-.144	1
471	8	max	151.324	4	357.659	3	95.043	1	0	2	.011	4	.045	1
472		min	9.054	10	-272.453	1	3.347	12	0	3	0	3	-.06	3
473	9	max	143.412	4	456.805	3	130.034	1	0	2	.092	1	.295	1
474		min	9.054	10	-348.033	1	4.879	12	0	3	.003	12	-.388	3
475	10	max	135.5	4	555.951	3	165.026	1	0	2	.211	1	.605	1
476		min	9.054	10	-423.613	1	6.411	12	0	3	.008	12	-.796	3
477	11	max	101.273	4	348.033	1	-1.127	15	0	3	.089	1	.295	1
478		min	4.453	12	-456.805	3	-129.355	1	0	2	-.013	5	-.388	3
479	12	max	93.361	4	272.453	1	.453	15	0	3	.002	2	.045	1
480		min	4.453	12	-357.659	3	-94.363	1	0	2	-.015	4	-.06	3
481	13	max	85.449	4	196.872	1	2.866	5	0	3	-.005	12	.188	3
482		min	4.453	12	-258.513	3	-59.372	1	0	2	-.063	1	-.144	1
483	14	max	79.805	1	121.292	1	5.31	5	0	3	-.006	12	.356	3
484		min	4.453	12	-159.367	3	-24.381	1	0	2	-.097	1	-.272	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	79.805	1	45.712	1	10.61	1	0	3	-.003	15	.445	3
486		min	4.453	12	-60.221	3	.734	10	0	2	-.103	1	-.34	1
487	16	max	79.805	1	38.925	3	45.602	1	0	3	.003	5	.453	3
488		min	4.453	12	-29.868	1	2.97	12	0	2	-.08	1	-.346	1
489	17	max	79.805	1	138.071	3	80.593	1	0	3	.012	5	.382	3
490		min	4.453	12	-105.448	1	4.503	12	0	2	-.029	1	-.292	1
491	18	max	79.805	1	237.217	3	115.584	1	0	3	.05	1	.231	3
492		min	4.453	12	-181.028	1	6.035	12	0	2	.004	12	-.176	1
493	19	max	79.805	1	336.363	3	150.575	1	0	3	.157	1	0	1
494		min	4.453	12	-256.608	1	7.567	12	0	2	.01	12	0	3
495	M16	1	max	51.491	5	360.337	2	-.657	15	0	.154	1	0	2
496		min	-79.998	1	-163.208	3	-149.779	1	0	2	-.026	5	0	3
497	2	max	43.579	5	254.222	2	1.061	5	0	3	.047	1	.112	3
498		min	-79.998	1	-115.276	3	-114.788	1	0	2	-.026	5	-.247	2
499	3	max	35.667	5	148.107	2	3.504	5	0	3	-.001	12	.186	3
500		min	-79.998	1	-67.344	3	-79.797	1	0	2	-.032	4	-.41	2
501	4	max	27.755	5	41.992	2	5.948	5	0	3	-.004	12	.221	3
502		min	-79.998	1	-19.412	3	-44.806	1	0	2	-.081	1	-.486	2
503	5	max	19.843	5	28.52	3	8.391	5	0	3	-.005	12	.217	3
504		min	-79.998	1	-64.123	2	-9.814	1	0	2	-.103	1	-.477	2
505	6	max	11.931	5	76.452	3	25.177	1	0	3	-.005	15	.175	3
506		min	-79.998	1	-170.238	2	.532	12	0	2	-.097	1	-.383	2
507	7	max	4.019	5	124.384	3	60.168	1	0	3	.002	5	.094	3
508		min	-79.998	1	-276.353	2	2.065	12	0	2	-.063	1	-.203	2
509	8	max	-1.73	12	172.316	3	95.159	1	0	3	.014	4	.062	2
510		min	-79.998	1	-382.468	2	3.597	12	0	2	-.003	3	-.026	3
511	9	max	-1.73	12	220.248	3	130.15	1	0	3	.091	1	.413	2
512		min	-79.998	1	-488.583	2	5.13	12	0	2	.002	12	-.184	3
513	10	max	28.208	5	-12.641	15	165.142	1	0	14	.21	1	.85	2
514		min	-81.716	1	-594.698	2	-10.323	3	0	2	.008	12	-.381	3
515	11	max	20.296	5	488.583	2	-1.102	15	0	2	.091	1	.413	2
516		min	-81.716	1	-220.248	3	-129.785	1	0	3	-.012	5	-.184	3
517	12	max	12.384	5	382.468	2	.477	15	0	2	.002	2	.062	2
518		min	-81.716	1	-172.316	3	-94.794	1	0	3	-.013	4	-.026	3
519	13	max	4.473	5	276.353	2	2.9	5	0	2	-.003	12	.094	3
520		min	-81.716	1	-124.384	3	-59.803	1	0	3	-.062	1	-.203	2
521	14	max	-2.164	15	170.238	2	5.343	5	0	2	-.004	12	.175	3
522		min	-81.716	1	-76.452	3	-24.812	1	0	3	-.096	1	-.383	2
523	15	max	-4.731	12	64.123	2	10.351	4	0	2	-.002	15	.217	3
524		min	-81.716	1	-28.52	3	.743	10	0	3	-.102	1	-.477	2
525	16	max	-4.731	12	19.412	3	45.171	1	0	2	.005	5	.221	3
526		min	-81.716	1	-41.992	2	2.281	12	0	3	-.08	1	-.486	2
527	17	max	-4.731	12	67.344	3	80.162	1	0	2	.014	5	.186	3
528		min	-81.716	1	-148.107	2	3.814	12	0	3	-.029	1	-.41	2
529	18	max	-4.731	12	115.276	3	115.153	1	0	2	.049	1	.112	3
530		min	-81.716	1	-254.222	2	5.346	12	0	3	.003	12	-.247	2
531	19	max	-4.731	12	163.208	3	150.144	1	0	2	.156	1	0	2
532		min	-81.716	1	-360.337	2	6.879	12	0	3	.008	12	0	3
533	M15	1	max	0	2	1.981	1	.042	3	0	1	0	1	1
534		min	-58.341	3	0	2	-.034	1	0	3	0	3	0	1
535	2	max	0	2	1.76	1	.042	3	0	1	0	1	0	2
536		min	-58.417	3	0	2	-.034	1	0	3	0	3	0	1
537	3	max	0	2	1.54	1	.042	3	0	1	0	1	0	2
538		min	-58.492	3	0	2	-.034	1	0	3	0	3	-.002	1
539	4	max	0	2	1.32	1	.042	3	0	1	0	1	0	2
540		min	-58.568	3	0	2	-.034	1	0	3	0	3	-.002	1
541	5	max	0	2	1.1	1	.042	3	0	1	0	1	0	2



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-58.643	3	0	2	-.034	1	0	3	0	3	-.003	1
543		6	max	0	2	.88	1	.042	3	0	1	0	1	0	2
544			min	-58.719	3	0	2	-.034	1	0	3	0	3	-.003	1
545		7	max	0	2	.66	1	.042	3	0	1	0	3	0	2
546			min	-58.794	3	0	2	-.034	1	0	3	0	1	-.004	1
547		8	max	0	2	.44	1	.042	3	0	1	0	3	0	2
548			min	-58.87	3	0	2	-.034	1	0	3	0	1	-.004	1
549		9	max	0	2	.22	1	.042	3	0	1	0	3	0	2
550			min	-58.946	3	0	2	-.034	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.042	3	0	1	0	3	0	2
552			min	-59.021	3	0	1	-.034	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.042	3	0	1	0	3	0	2
554			min	-59.097	3	-.22	1	-.034	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.042	3	0	1	0	3	0	2
556			min	-59.172	3	-.44	1	-.034	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.042	3	0	1	0	3	0	2
558			min	-59.248	3	-.66	1	-.034	1	0	3	0	1	-.004	1
559		14	max	0	2	0	2	.042	3	0	1	0	3	0	2
560			min	-59.323	3	-.88	1	-.034	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.042	3	0	1	0	3	0	2
562			min	-59.399	3	-1.1	1	-.034	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.042	3	0	1	0	3	0	2
564			min	-59.474	3	-1.32	1	-.034	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.042	3	0	1	0	3	0	2
566			min	-59.55	3	-1.54	1	-.034	1	0	3	0	1	-.002	1
567		18	max	0	2	0	2	.042	3	0	1	0	3	0	2
568			min	-59.625	3	-1.76	1	-.034	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.042	3	0	1	0	3	0	1
570			min	-59.701	3	-1.981	1	-.034	1	0	3	0	1	0	1
571	M16A	1	max	-.985	10	3.318	4	.316	4	0	3	0	3	0	1
572			min	-245.238	4	.99	12	-.017	3	0	2	0	4	0	1
573		2	max	-.901	10	2.949	4	.284	4	0	3	0	3	0	12
574			min	-245.297	4	.88	12	-.017	3	0	2	0	4	-.001	4
575		3	max	-.817	10	2.581	4	.252	4	0	3	0	3	0	12
576			min	-245.357	4	.77	12	-.017	3	0	2	0	4	-.003	4
577		4	max	-.733	10	2.212	4	.22	4	0	3	0	3	-.001	12
578			min	-245.416	4	.66	12	-.017	3	0	2	0	4	-.004	4
579		5	max	-.649	10	1.843	4	.189	4	0	3	0	3	-.001	12
580			min	-245.475	4	.55	12	-.017	3	0	2	0	1	-.005	4
581		6	max	-.565	10	1.475	4	.157	4	0	3	0	5	-.002	12
582			min	-245.535	4	.44	12	-.017	3	0	2	0	1	-.005	4
583		7	max	-.481	10	1.106	4	.125	4	0	3	0	5	-.002	12
584			min	-245.594	4	.33	12	-.017	3	0	2	0	1	-.006	4
585		8	max	-.397	10	.737	4	.093	4	0	3	0	5	-.002	12
586			min	-245.653	4	.22	12	-.017	3	0	2	0	1	-.006	4
587		9	max	-.313	10	.369	4	.061	4	0	3	0	5	-.002	12
588			min	-245.712	4	.11	12	-.017	3	0	2	0	1	-.007	4
589		10	max	-.229	10	0	1	.029	4	0	3	0	5	-.002	12
590			min	-245.772	4	0	1	-.017	3	0	2	0	1	-.007	4
591		11	max	-.145	10	-.11	12	.021	1	0	3	0	5	-.002	12
592			min	-245.831	4	-.369	4	-.017	3	0	2	0	1	-.007	4
593		12	max	-.061	10	-.22	12	.021	1	0	3	0	5	-.002	12
594			min	-245.89	4	-.737	4	-.038	5	0	2	0	1	-.006	4
595		13	max	.023	10	-.33	12	.021	1	0	3	0	5	-.002	12
596			min	-245.949	4	-1.106	4	-.07	5	0	2	0	3	-.006	4
597		14	max	.106	10	-.44	12	.021	1	0	3	0	4	-.002	12
598			min	-246.009	4	-1.475	4	-.102	5	0	2	0	3	-.005	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.19	10	-.55	12	.021	1	0	3	0	4	-.001	12
600		min	-246.068	4	-1.843	4	-.134	5	0	2	0	3	-.005	4
601	16	max	.274	10	-.66	12	.021	1	0	3	0	4	-.001	12
602		min	-246.127	4	-2.212	4	-.166	5	0	2	0	3	-.004	4
603	17	max	.358	10	-.77	12	.021	1	0	3	0	1	0	12
604		min	-246.186	4	-2.581	4	-.198	5	0	2	0	5	-.003	4
605	18	max	.442	10	-.88	12	.021	1	0	3	0	1	0	12
606		min	-246.246	4	-2.949	4	-.23	5	0	2	0	5	-.001	4
607	19	max	.526	10	-.99	12	.021	1	0	3	0	1	0	1
608		min	-246.305	4	-3.318	4	-.261	5	0	2	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.01	2	.015	1	1.859e-3	5	NC	3	NC	3	
2			min	-.004	3	-.011	3	-.017	5	-1.323e-3	1	4084.842	2	2787.788	1	
3			2	max	.003	1	.009	2	.014	1	1.882e-3	5	NC	3	NC	3
4				min	-.004	3	-.01	3	-.017	5	-1.265e-3	1	4472.134	2	2991.38	1
5			3	max	.002	1	.009	2	.013	1	1.905e-3	5	NC	1	NC	3
6				min	-.003	3	-.01	3	-.017	5	-1.206e-3	1	4935.4	2	3232.632	1
7			4	max	.002	1	.008	2	.012	1	1.927e-3	5	NC	1	NC	3
8				min	-.003	3	-.01	3	-.016	5	-1.148e-3	1	5493.442	2	3520.369	1
9			5	max	.002	1	.007	2	.011	1	1.95e-3	5	NC	1	NC	3
10				min	-.003	3	-.009	3	-.016	5	-1.089e-3	1	6171.452	2	3866.374	1
11			6	max	.002	1	.006	2	.01	1	1.972e-3	5	NC	1	NC	2
12				min	-.003	3	-.009	3	-.015	5	-1.031e-3	1	7003.75	2	4286.654	1
13			7	max	.002	1	.005	2	.009	1	1.995e-3	5	NC	1	NC	2
14				min	-.003	3	-.008	3	-.015	5	-9.726e-4	1	8037.982	2	4803.395	1
15			8	max	.002	1	.005	2	.008	1	2.018e-3	5	NC	1	NC	2
16				min	-.002	3	-.008	3	-.014	5	-9.142e-4	1	9341.778	2	5448.071	1
17			9	max	.002	1	.004	2	.007	1	2.04e-3	5	NC	1	NC	2
18				min	-.002	3	-.007	3	-.013	5	-8.557e-4	1	NC	1	6266.573	1
19			10	max	.001	1	.003	2	.006	1	2.063e-3	5	NC	1	NC	2
20				min	-.002	3	-.007	3	-.012	5	-7.973e-4	1	NC	1	7328.032	1
21		11	max	.001	1	.003	2	.005	1	2.085e-3	5	NC	1	NC	2	
22			min	-.002	3	-.006	3	-.011	5	-7.388e-4	1	NC	1	8740.742	1	
23		12	max	.001	1	.002	2	.004	1	2.108e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.01	5	-6.804e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.003	1	2.13e-3	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.009	5	-6.219e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.153e-3	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.008	5	-5.635e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.176e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.007	5	-5.05e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	2.198e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.005	5	-4.466e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.221e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.003	5	-3.881e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.243e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.002	5	-3.297e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.266e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.712e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.299e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.084e-3	5	NC	1	NC	1	
41			2	max	0	3	0	2	.005	5	1.575e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-1.102e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.011	5	1.852e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.119e-3	5	NC	1	8999.493	14
45		4	max	0	3	0	2	.016	5	2.128e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	-.001	1	-1.137e-3	5	NC	1	5889.295	14
47		5	max	0	3	0	2	.022	5	2.404e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	-1.154e-3	5	NC	1	4346.507	14
49		6	max	0	3	0	2	.027	5	2.68e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.172e-3	5	NC	1	3429.394	14
51		7	max	0	3	0	2	.033	4	2.956e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.19e-3	5	NC	1	2824.213	14
53		8	max	0	3	0	2	.038	4	3.233e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.207e-3	5	NC	1	2396.618	14
55		9	max	.001	3	.001	2	.044	4	3.509e-4	1	NC	1	NC	1
56			min	0	2	-.007	3	0	11	-1.225e-3	5	NC	1	2079.512	14
57		10	max	.001	3	.002	2	.049	4	3.785e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-1.242e-3	5	NC	1	1835.675	14
59		11	max	.001	3	.002	2	.054	4	4.061e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-1.26e-3	5	NC	1	1642.813	14
61		12	max	.001	3	.003	2	.06	4	4.338e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	12	-1.278e-3	5	NC	1	1486.765	14
63		13	max	.002	3	.004	2	.065	4	4.614e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	12	-1.295e-3	5	NC	1	1358.11	14
65		14	max	.002	3	.004	2	.07	4	4.89e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	12	-1.313e-3	5	NC	1	1250.339	14
67		15	max	.002	3	.005	2	.075	4	5.166e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	12	-1.33e-3	5	8804.49	2	1158.812	14
69		16	max	.002	3	.006	2	.08	4	5.442e-4	1	NC	1	NC	2
70			min	-.002	2	-.009	3	0	12	-1.348e-3	5	7449.056	2	1080.133	14
71		17	max	.002	3	.007	2	.085	4	5.719e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	12	-1.366e-3	5	6403.576	2	1011.758	14
73		18	max	.002	3	.008	2	.09	4	5.995e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	12	-1.383e-3	5	5587.31	2	951.743	14
75		19	max	.002	3	.009	2	.095	4	6.271e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	12	-1.401e-3	5	4943.906	2	898.58	14
77	M4	1	max	.002	1	.012	2	0	12	7.676e-3	5	NC	1	NC	3
78			min	0	15	-.011	3	-.1	4	-1.009e-3	1	NC	1	193.084	4
79		2	max	.002	1	.012	2	0	12	7.676e-3	5	NC	1	NC	3
80			min	0	15	-.01	3	-.092	4	-1.009e-3	1	NC	1	210.488	4
81		3	max	.002	1	.011	2	0	12	7.676e-3	5	NC	1	NC	3
82			min	0	15	-.01	3	-.084	4	-1.009e-3	1	NC	1	231.203	4
83		4	max	.002	1	.01	2	0	12	7.676e-3	5	NC	1	NC	2
84			min	0	15	-.009	3	-.075	4	-1.009e-3	1	NC	1	256.101	4
85		5	max	.001	1	.01	2	0	12	7.676e-3	5	NC	1	NC	2
86			min	0	15	-.008	3	-.067	4	-1.009e-3	1	NC	1	286.372	4
87		6	max	.001	1	.009	2	0	12	7.676e-3	5	NC	1	NC	2
88			min	0	15	-.008	3	-.06	4	-1.009e-3	1	NC	1	323.666	4
89		7	max	.001	1	.008	2	0	12	7.676e-3	5	NC	1	NC	2
90			min	0	15	-.007	3	-.052	4	-1.009e-3	1	NC	1	370.338	4
91		8	max	.001	1	.007	2	0	12	7.676e-3	5	NC	1	NC	2
92			min	0	15	-.007	3	-.045	4	-1.009e-3	1	NC	1	429.833	4
93		9	max	.001	1	.007	2	0	12	7.676e-3	5	NC	1	NC	2
94			min	0	15	-.006	3	-.038	4	-1.009e-3	1	NC	1	507.364	4
95		10	max	0	1	.006	2	0	12	7.676e-3	5	NC	1	NC	1
96			min	0	15	-.005	3	-.032	4	-1.009e-3	1	NC	1	611.123	4
97		11	max	0	1	.005	2	0	12	7.676e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.026	4	-1.009e-3	1	NC	1	754.606	4
99		12	max	0	1	.005	2	0	12	7.676e-3	5	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.02	4	-1.009e-3	1	NC	1	961.351	4
101		max	0	1	.004	2	0	12	7.676e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.015	4	-1.009e-3	1	NC	1	1275.51	4
103		max	0	1	.003	2	0	12	7.676e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.011	4	-1.009e-3	1	NC	1	1788.158	4
105		max	0	1	.003	2	0	12	7.676e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.007	4	-1.009e-3	1	NC	1	2713.262	4
107		max	0	1	.002	2	0	12	7.676e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.004	4	-1.009e-3	1	NC	1	4659.085	4
109		max	0	1	.001	2	0	12	7.676e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.002	4	-1.009e-3	1	NC	1	9987.994	4
111		max	0	1	0	2	0	12	7.676e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-1.009e-3	1	NC	1	NC	1
113		max	0	1	0	1	0	1	7.676e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-1.009e-3	1	NC	1	NC	1
115	M6	max	.009	1	.038	2	.005	1	2.046e-3	4	NC	3	NC	2
116		min	-.012	3	-.035	3	-.018	5	8.892e-7	10	1130.202	2	8484.385	1
117		max	.008	1	.035	2	.005	1	2.065e-3	4	NC	3	NC	2
118		min	-.012	3	-.033	3	-.017	5	1.456e-7	10	1209.878	2	9197.586	1
119		max	.008	1	.033	2	.004	1	2.083e-3	4	NC	3	NC	1
120		min	-.011	3	-.031	3	-.017	5	-1.04e-6	2	1301.234	2	NC	1
121		max	.007	1	.03	2	.004	1	2.101e-3	4	NC	3	NC	1
122		min	-.01	3	-.029	3	-.017	5	-3.858e-6	2	1406.597	2	NC	1
123		max	.007	1	.028	2	.003	1	2.12e-3	4	NC	3	NC	1
124		min	-.01	3	-.027	3	-.016	5	-6.677e-6	2	1528.963	2	NC	1
125		max	.006	1	.025	2	.003	1	2.138e-3	4	NC	3	NC	1
126		min	-.009	3	-.026	3	-.016	5	-9.495e-6	2	1672.252	2	NC	1
127		max	.006	1	.023	2	.003	1	2.157e-3	4	NC	3	NC	1
128		min	-.008	3	-.024	3	-.015	5	-1.231e-5	2	1841.693	2	NC	1
129		max	.005	1	.021	2	.002	1	2.175e-3	4	NC	3	NC	1
130		min	-.007	3	-.022	3	-.015	5	-1.513e-5	2	2044.414	2	NC	1
131		max	.005	1	.019	2	.002	1	2.193e-3	4	NC	3	NC	1
132		min	-.007	3	-.02	3	-.014	5	-1.795e-5	2	2290.387	2	9580.856	4
133		max	.004	1	.016	2	.002	1	2.212e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.013	5	-2.077e-5	2	2594.013	2	9352.162	4
135		max	.004	1	.014	2	.001	1	2.23e-3	4	NC	3	NC	1
136		min	-.005	3	-.016	3	-.012	5	-2.359e-5	2	2976.875	2	9388.664	4
137		max	.003	1	.012	2	.001	1	2.248e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.011	5	-2.641e-5	2	3472.865	2	9701.946	4
139		max	.003	1	.01	2	0	1	2.267e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.01	5	-2.922e-5	2	4138.441	2	NC	1
141		max	.002	1	.008	2	0	1	2.285e-3	4	NC	3	NC	1
142		min	-.003	3	-.01	3	-.008	5	-3.204e-5	2	5075.169	2	NC	1
143		max	.002	1	.007	2	0	1	2.303e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.007	5	-3.486e-5	2	6486.106	2	NC	1
145		max	.001	1	.005	2	0	1	2.322e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.005	5	-3.768e-5	2	8844.924	2	NC	1
147		max	0	1	.003	2	0	1	2.34e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.004	5	-4.05e-5	2	NC	1	NC	1
149		max	0	1	.002	2	0	1	2.36e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-4.332e-5	2	NC	1	NC	1
151		max	0	1	0	1	0	1	2.38e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.349e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.522e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.138e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.006	5	2.347e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-1.142e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.011	5	2.172e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.146e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.017	5	1.997e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.151e-3	4	9965.3	2	NC	1
161		5	max	.002	3	.006	2	.023	5	1.823e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-1.156e-3	4	7518.677	2	NC	1
163		6	max	.002	3	.008	2	.029	5	2.443e-5	3	NC	3	NC	1
164			min	-.002	2	-.01	3	0	1	-1.16e-3	4	6021.548	2	NC	1
165		7	max	.003	3	.009	2	.034	5	4.274e-5	3	NC	3	NC	1
166			min	-.003	2	-.012	3	0	1	-1.165e-3	4	5001.43	2	NC	1
167		8	max	.003	3	.011	2	.04	5	6.105e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-1.17e-3	4	4256.307	2	NC	1
169		9	max	.003	3	.012	2	.045	5	7.936e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	-.001	1	-1.174e-3	4	3685.427	2	NC	1
171		10	max	.004	3	.014	2	.05	5	9.767e-5	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-1.179e-3	4	3232.9	2	NC	1
173		11	max	.004	3	.016	2	.056	4	1.16e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.183e-3	4	2865.17	2	NC	1
175		12	max	.005	3	.018	2	.061	4	1.343e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.002	1	-1.188e-3	4	2560.787	2	NC	1
177		13	max	.005	3	.02	2	.066	4	1.526e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.193e-3	4	2305.323	2	NC	1
179		14	max	.006	3	.022	2	.071	4	1.709e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-1.197e-3	4	2088.649	2	NC	1
181		15	max	.006	3	.024	2	.076	4	1.892e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.002	1	-1.202e-3	4	1903.395	2	NC	1
183		16	max	.006	3	.026	2	.081	4	2.075e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.002	1	-1.207e-3	4	1744.034	2	NC	1
185		17	max	.007	3	.029	2	.085	4	2.258e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.002	1	-1.211e-3	4	1606.321	2	NC	1
187		18	max	.007	3	.031	2	.09	4	2.441e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-1.216e-3	4	1486.927	2	NC	1
189		19	max	.008	3	.033	2	.095	4	2.625e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-1.221e-3	4	1383.202	2	NC	1
191	M8	1	max	.005	1	.044	2	.003	1	7.504e-3	4	NC	1	NC	2
192			min	0	15	-.034	3	-.1	4	-2.128e-4	3	NC	1	193.472	4
193		2	max	.004	1	.041	2	.002	1	7.504e-3	4	NC	1	NC	2
194			min	0	15	-.032	3	-.092	4	-2.128e-4	3	NC	1	210.91	4
195		3	max	.004	1	.039	2	.002	1	7.504e-3	4	NC	1	NC	2
196			min	0	15	-.031	3	-.083	4	-2.128e-4	3	NC	1	231.665	4
197		4	max	.004	1	.036	2	.002	1	7.504e-3	4	NC	1	NC	1
198			min	0	15	-.029	3	-.075	4	-2.128e-4	3	NC	1	256.613	4
199		5	max	.004	1	.034	2	.002	1	7.504e-3	4	NC	1	NC	1
200			min	0	15	-.027	3	-.067	4	-2.128e-4	3	NC	1	286.942	4
201		6	max	.003	1	.031	2	.002	1	7.504e-3	4	NC	1	NC	1
202			min	0	15	-.025	3	-.06	4	-2.128e-4	3	NC	1	324.309	4
203		7	max	.003	1	.029	2	.001	1	7.504e-3	4	NC	1	NC	1
204			min	0	15	-.023	3	-.052	4	-2.128e-4	3	NC	1	371.072	4
205		8	max	.003	1	.027	2	.001	1	7.504e-3	4	NC	1	NC	1
206			min	0	15	-.021	3	-.045	4	-2.128e-4	3	NC	1	430.683	4
207		9	max	.003	1	.024	2	0	1	7.504e-3	4	NC	1	NC	1
208			min	0	15	-.019	3	-.038	4	-2.128e-4	3	NC	1	508.365	4
209		10	max	.002	1	.022	2	0	1	7.504e-3	4	NC	1	NC	1
210			min	0	15	-.017	3	-.032	4	-2.128e-4	3	NC	1	612.327	4
211		11	max	.002	1	.019	2	0	1	7.504e-3	4	NC	1	NC	1
212			min	0	15	-.015	3	-.026	4	-2.128e-4	3	NC	1	756.089	4
213		12	max	.002	1	.017	2	0	1	7.504e-3	4	NC	1	NC	1







Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.018	5	9.986e-5	10	NC	1	1052.182	5
329		max	0	1	.004	2	.002	1	9.001e-3	4	NC	1	NC	1
330		min	0	15	-.004	3	-.014	5	9.986e-5	10	NC	1	1395.979	5
331		max	0	1	.003	2	.001	1	9.001e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.01	5	9.986e-5	10	NC	1	1956.982	5
333		max	0	1	.003	2	0	1	9.001e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.007	5	9.986e-5	10	NC	1	2969.328	5
335		max	0	1	.002	2	0	1	9.001e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.004	5	9.986e-5	10	NC	1	5098.615	5
337		max	0	1	.001	2	0	1	9.001e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.002	5	9.986e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	9.001e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	9.986e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	9.001e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	9.986e-5	10	NC	1	NC	1
343	M1	max	.009	3	.026	3	.009	5	1.853e-2	1	NC	1	NC	1
344		min	-.009	2	-.024	2	-.005	1	-2.414e-2	3	NC	1	NC	1
345		max	.009	3	.016	3	.013	5	8.776e-3	1	NC	4	NC	2
346		min	-.009	2	-.014	2	-.011	1	-1.196e-2	3	4924.142	2	7644.944	1
347		max	.009	3	.007	3	.018	5	6.814e-4	5	NC	4	NC	2
348		min	-.009	2	-.005	2	-.015	1	-7.915e-4	1	2527.671	2	4638.269	1
349		max	.009	3	.003	1	.022	5	7.03e-4	5	NC	4	NC	3
350		min	-.009	2	-.002	3	-.017	1	-6.845e-4	1	1768.448	2	3637.78	5
351		max	.009	3	.01	2	.027	5	7.246e-4	5	NC	4	NC	3
352		min	-.009	2	-.008	3	-.018	1	-5.775e-4	1	1401.684	2	2603.256	5
353		max	.009	3	.016	2	.033	5	7.461e-4	5	NC	5	NC	2
354		min	-.009	2	-.014	3	-.017	1	-4.705e-4	1	1192.921	2	1999.675	5
355		max	.009	3	.02	2	.039	5	7.677e-4	5	NC	5	NC	2
356		min	-.009	2	-.018	3	-.015	1	-3.635e-4	1	1065.135	2	1608.973	5
357		max	.009	3	.024	2	.044	5	7.893e-4	5	NC	5	NC	2
358		min	-.009	2	-.021	3	-.012	1	-2.566e-4	1	986.266	2	1338.255	5
359		max	.009	3	.026	2	.05	5	8.109e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.008	1	-1.496e-4	1	941.363	2	1139.548	4
361		max	.009	3	.027	2	.057	5	8.325e-4	5	NC	5	NC	1
362		min	-.009	2	-.023	3	-.005	1	-4.26e-5	1	923.609	2	972.319	4
363		max	.009	3	.027	2	.063	4	8.702e-4	4	NC	5	NC	1
364		min	-.009	2	-.022	3	-.001	1	1.405e-5	10	931.146	2	847.293	4
365		max	.009	3	.025	2	.07	4	9.176e-4	4	NC	5	NC	2
366		min	-.009	2	-.021	3	0	10	2.571e-5	10	966.419	2	751.669	4
367		max	.009	3	.022	2	.077	4	9.651e-4	4	NC	5	NC	2
368		min	-.009	2	-.018	3	0	12	3.197e-5	12	1037.498	2	677.29	4
369		max	.009	3	.017	2	.083	4	1.013e-3	4	NC	4	NC	2
370		min	-.009	2	-.013	3	0	12	3.64e-5	12	1162.786	2	618.77	4
371		max	.009	3	.01	2	.089	4	1.06e-3	4	NC	4	NC	3
372		min	-.009	2	-.008	3	0	12	4.082e-5	12	1385.218	2	572.444	4
373		max	.009	3	.002	1	.095	4	1.468e-3	4	NC	4	NC	3
374		min	-.009	2	-.002	3	0	12	4.396e-5	12	1824.001	2	535.77	4
375		max	.009	3	.006	3	.1	4	1.044e-2	4	NC	4	NC	2
376		min	-.009	2	-.008	2	0	12	-1.821e-4	1	2573.052	1	506.985	4
377		max	.009	3	.014	3	.104	4	1.286e-2	2	NC	2	NC	2
378		min	-.009	2	-.019	2	0	10	-5.95e-3	3	4973.31	1	484.784	4
379		max	.009	3	.022	3	.108	4	2.604e-2	2	NC	1	NC	1
380		min	-.009	2	-.032	2	-.004	1	-1.204e-2	3	5950.696	2	468.839	4
381	M5	max	.03	3	.086	3	.009	5	9.678e-6	4	NC	1	NC	1
382		min	-.034	2	-.08	2	-.006	1	5.091e-8	10	3560.161	3	NC	1
383		max	.03	3	.052	3	.013	5	3.467e-4	5	NC	5	NC	1
384		min	-.034	2	-.048	2	-.006	1	-5.483e-5	1	1465.845	2	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.03	3	.021	3	.018	5	6.781e-4	5	NC	5	NC	1
386		min	-.034	2	-.018	2	-.005	1	-1.095e-4	1	751.991	2	NC	1
387	4	max	.03	3	.009	2	.023	5	7.081e-4	5	NC	5	NC	1
388		min	-.034	2	-.005	3	-.004	1	-1.048e-4	1	525.553	2	NC	1
389	5	max	.03	3	.033	2	.029	5	7.381e-4	5	NC	5	NC	1
390		min	-.034	2	-.027	3	-.004	1	-1.002e-4	1	416.118	2	NC	1
391	6	max	.03	3	.053	2	.034	5	7.681e-4	5	NC	15	NC	1
392		min	-.034	2	-.045	3	-.004	1	-9.556e-5	1	353.794	2	NC	1
393	7	max	.029	3	.069	2	.041	5	7.981e-4	5	NC	15	NC	1
394		min	-.034	2	-.059	3	-.003	1	-9.093e-5	1	315.612	2	NC	1
395	8	max	.029	3	.081	2	.047	5	8.281e-4	5	NC	15	NC	1
396		min	-.034	2	-.068	3	-.003	1	-8.629e-5	1	292.009	2	NC	1
397	9	max	.029	3	.088	2	.053	5	8.581e-4	5	NC	15	NC	1
398		min	-.034	2	-.074	3	-.003	1	-8.165e-5	1	278.522	2	NC	1
399	10	max	.029	3	.092	2	.06	5	8.882e-4	5	NC	15	NC	1
400		min	-.034	2	-.075	3	-.003	1	-7.702e-5	1	273.114	2	NC	1
401	11	max	.029	3	.09	2	.066	5	9.182e-4	5	NC	15	NC	1
402		min	-.034	2	-.073	3	-.003	1	-7.238e-5	1	275.227	2	NC	1
403	12	max	.029	3	.084	2	.072	5	9.482e-4	5	NC	15	NC	1
404		min	-.034	2	-.067	3	-.002	1	-6.775e-5	1	285.585	2	NC	1
405	13	max	.029	3	.073	2	.078	5	9.782e-4	5	NC	15	NC	1
406		min	-.034	2	-.057	3	-.002	1	-6.311e-5	1	306.593	2	NC	1
407	14	max	.029	3	.056	2	.084	5	1.008e-3	5	NC	15	NC	1
408		min	-.034	2	-.043	3	-.002	1	-5.847e-5	1	343.754	2	NC	1
409	15	max	.029	3	.034	2	.09	4	1.038e-3	5	NC	5	NC	1
410		min	-.034	2	-.026	3	-.002	1	-5.384e-5	1	409.953	2	NC	1
411	16	max	.028	3	.007	1	.095	4	1.428e-3	5	NC	5	NC	1
412		min	-.034	2	-.006	3	-.002	1	-5.733e-5	1	541.216	2	NC	1
413	17	max	.028	3	.019	3	.1	4	1.042e-2	4	NC	5	NC	1
414		min	-.034	2	-.027	2	-.003	1	-2.543e-4	1	868.16	3	NC	1
415	18	max	.028	3	.045	3	.104	4	5.342e-3	4	NC	5	NC	1
416		min	-.034	2	-.066	2	-.003	1	-1.302e-4	1	1702.75	3	NC	1
417	19	max	.028	3	.073	3	.108	4	2.236e-6	5	NC	3	NC	1
418		min	-.034	2	-.107	2	-.003	1	-3.901e-7	3	1707.929	2	NC	1
419	M9	1	max	.009	.026	.007	.007	5	2.414e-2	3	NC	1	NC	1
420		min	-.009	2	-.024	2	-.007	1	-1.852e-2	1	NC	1	NC	1
421	2	max	.009	3	.016	.007	.007	5	1.193e-2	3	NC	4	NC	2
422		min	-.009	2	-.014	2	-.001	1	-9.015e-3	1	4925.585	2	8657.882	1
423	3	max	.009	3	.006	.008	.008	4	3.151e-4	1	NC	4	NC	2
424		min	-.009	2	-.005	2	0	3	-5.6e-5	3	2528.431	2	5355.385	1
425	4	max	.009	3	.003	.01	.01	4	2.247e-4	1	NC	4	NC	2
426		min	-.009	2	-.002	3	0	3	-6.122e-5	3	1768.98	2	4521.531	1
427	5	max	.009	3	.01	.013	.013	4	1.343e-4	1	NC	4	NC	3
428		min	-.009	2	-.009	3	-.002	3	-6.644e-5	3	1402.087	2	4459.803	1
429	6	max	.009	3	.016	.016	.016	4	1.084e-4	4	NC	4	NC	2
430		min	-.009	2	-.014	3	-.002	3	-7.166e-5	3	1193.241	2	4232.723	4
431	7	max	.009	3	.02	.02	.02	4	1.229e-4	5	NC	5	NC	2
432		min	-.009	2	-.018	3	-.003	3	-7.688e-5	3	1065.396	2	3087.073	4
433	8	max	.009	3	.024	.025	.025	4	1.565e-4	5	NC	5	NC	1
434		min	-.009	2	-.021	3	-.003	3	-1.369e-4	1	986.481	2	2356.912	4
435	9	max	.009	3	.026	.031	.031	5	1.902e-4	5	NC	5	NC	1
436		min	-.009	2	-.023	3	-.004	3	-2.273e-4	1	941.538	2	1863.417	4
437	10	max	.009	3	.027	.037	.037	5	2.239e-4	5	NC	5	NC	1
438		min	-.009	2	-.023	3	-.006	1	-3.177e-4	1	923.744	2	1514.283	4
439	11	max	.009	3	.027	.044	.044	5	2.576e-4	5	NC	5	NC	2
440		min	-.009	2	-.023	3	-.01	1	-4.081e-4	1	931.239	2	1258.108	4
441	12	max	.009	3	.025	.051	.051	5	2.913e-4	5	NC	5	NC	2





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Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.227	3	.116	1	7.067e-3	2	NC	5	NC	3
500		min	-.108	4	-.474	2	.008	10	-4.849e-3	3	393.485	2	1439.645	1
501	4	max	.002	1	.286	3	.175	1	8.292e-3	2	NC	15	NC	12
502		min	-.108	4	-.599	2	.013	10	-5.654e-3	3	306.748	2	967.163	1
503	5	max	.002	1	.305	3	.204	1	9.517e-3	2	NC	15	9923.097	12
504		min	-.108	4	-.636	2	.015	10	-6.458e-3	3	287.966	2	833.689	1
505	6	max	.002	1	.285	3	.194	1	1.074e-2	2	NC	15	NC	15
506		min	-.108	4	-.586	2	.012	10	-7.263e-3	3	313.611	2	875.155	1
507	7	max	.002	1	.233	3	.148	1	1.197e-2	2	NC	5	NC	5
508		min	-.108	4	-.468	2	.005	10	-8.067e-3	3	398.751	2	1140.106	1
509	8	max	.003	1	.165	3	.079	1	1.319e-2	2	NC	5	NC	5
510		min	-.108	4	-.313	2	-.005	10	-8.872e-3	3	617.223	2	2088.761	1
511	9	max	.003	1	.102	3	.032	3	1.442e-2	2	NC	5	NC	1
512		min	-.108	4	-.172	2	-.02	2	-9.676e-3	3	1240.746	2	7613.91	3
513	10	max	.003	1	.073	3	.028	3	1.564e-2	2	NC	4	NC	4
514		min	-.108	4	-.107	2	-.034	2	-1.048e-2	3	2294.212	2	7168.635	2
515	11	max	.003	1	.102	3	.028	3	1.442e-2	2	NC	5	NC	1
516		min	-.108	4	-.172	2	-.02	2	-9.675e-3	3	1240.746	2	8980.524	3
517	12	max	.003	1	.165	3	.077	1	1.319e-2	2	NC	5	NC	5
518		min	-.108	4	-.313	2	-.005	10	-8.869e-3	3	617.223	2	2130.928	1
519	13	max	.003	1	.233	3	.146	1	1.197e-2	2	NC	5	NC	5
520		min	-.108	4	-.468	2	.005	10	-8.064e-3	3	398.751	2	1158.771	1
521	14	max	.003	1	.285	3	.191	1	1.074e-2	2	NC	15	NC	5
522		min	-.108	4	-.586	2	.007	15	-7.258e-3	3	313.611	2	889.053	1
523	15	max	.003	1	.305	3	.201	1	9.518e-3	2	NC	15	NC	5
524		min	-.108	4	-.636	2	.003	15	-6.453e-3	3	287.966	2	847.845	1
525	16	max	.003	1	.286	3	.172	1	8.293e-3	2	NC	15	NC	3
526		min	-.108	4	-.599	2	-.003	5	-5.647e-3	3	306.748	2	986.214	1
527	17	max	.003	1	.227	3	.113	1	7.068e-3	2	NC	5	NC	3
528		min	-.108	4	-.474	2	-.009	5	-4.842e-3	3	393.485	2	1475.87	1
529	18	max	.003	1	.135	3	.044	1	5.843e-3	2	NC	5	NC	3
530		min	-.108	4	-.275	2	-.009	5	-4.036e-3	3	715.483	2	3550.159	1
531	19	max	.004	1	.022	3	.009	3	4.618e-3	2	NC	1	NC	1
532		min	-.108	4	-.032	2	-.009	2	-3.231e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.97e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-7.353e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.013	4	9.16e-4	3	NC	5	NC	1
536		min	0	5	-.015	1	0	3	-7.635e-4	5	6652.208	2	7601.434	4
537	3	max	0	3	0	15	.029	4	1.435e-3	3	NC	5	NC	1
538		min	-.002	5	-.029	1	-.004	3	-1.145e-3	2	3385.082	2	3375.56	4
539	4	max	0	3	0	15	.046	4	1.954e-3	3	NC	5	NC	9
540		min	-.003	5	-.042	1	-.008	3	-1.688e-3	2	2322.364	2	2115.099	4
541	5	max	0	3	0	15	.062	4	2.473e-3	3	NC	5	NC	9
542		min	-.004	5	-.054	1	-.013	3	-2.23e-3	2	1812.164	2	1554.862	4
543	6	max	0	3	-.001	15	.077	4	2.992e-3	3	NC	5	NC	9
544		min	-.004	5	-.064	1	-.019	3	-2.773e-3	2	1525.128	2	1261.814	4
545	7	max	0	3	-.001	15	.088	4	3.511e-3	3	NC	5	7970.524	9
546		min	-.005	5	-.072	1	-.025	3	-3.315e-3	2	1352.513	2	1099.7	4
547	8	max	0	3	-.001	15	.095	4	4.03e-3	3	NC	5	6648.683	9
548		min	-.006	5	-.078	1	-.031	3	-3.858e-3	2	1248.918	2	1014.891	4
549	9	max	0	3	0	15	.098	4	4.549e-3	3	NC	5	5776.045	9
550		min	-.007	5	-.081	1	-.036	3	-4.4e-3	2	1193.158	2	984.817	4
551	10	max	0	3	0	15	.096	4	5.068e-3	3	NC	5	5198.399	9
552		min	-.008	5	-.083	1	-.04	3	-4.943e-3	2	1175.518	2	1002.209	4
553	11	max	0	3	0	15	.09	4	5.587e-3	3	NC	5	4834.3	9
554		min	-.009	5	-.082	1	-.043	3	-5.485e-3	2	1193.158	2	1070.782	4
555	12	max	0	3	0	15	.08	4	6.106e-3	3	NC	5	4644.058	9



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Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.01	5	-.078	1	-.044	3	-6.028e-3	2	1248.918	2	1207.143	4
557	13	max	-.001	3	0	15	.066	4	6.625e-3	3	NC	5	4618.439	9
558		min	-.011	5	-.072	1	-.042	3	-6.571e-3	2	1352.513	2	1446.708	3
559	14	max	.001	3	.001	15	.051	4	7.143e-3	3	NC	5	4780.713	9
560		min	-.012	5	-.064	1	-.038	3	-7.113e-3	2	1525.128	2	1492.595	3
561	15	max	.001	3	.002	15	.034	4	7.662e-3	3	NC	5	5224.148	15
562		min	-.013	5	-.054	1	-.031	3	-7.656e-3	2	1812.164	2	1621.276	3
563	16	max	.001	3	.003	5	.02	1	8.181e-3	3	NC	5	NC	15
564		min	-.013	5	-.042	1	-.02	3	-8.198e-3	2	2322.364	2	1895.899	3
565	17	max	.001	3	.004	5	.008	1	8.7e-3	3	NC	5	NC	4
566		min	-.014	5	-.029	1	-.006	3	-8.741e-3	2	3385.082	2	2514.451	3
567	18	max	.001	3	.006	5	.013	3	9.219e-3	3	NC	5	NC	4
568		min	-.015	5	-.015	1	-.016	2	-9.283e-3	2	6652.208	2	4478.342	3
569	19	max	.002	3	.007	5	.037	3	9.738e-3	3	NC	1	NC	1
570		min	-.016	5	-.003	9	-.038	2	-9.826e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.011	3	2.869e-3	3	NC	1	NC	1
572		min	-.006	4	-.005	4	-.011	2	-2.689e-3	2	NC	1	NC	1
573	2	max	0	10	-.008	12	.004	9	2.756e-3	3	NC	12	NC	2
574		min	-.006	4	-.029	4	-.005	5	-2.572e-3	2	3970.646	4	9769.173	1
575	3	max	0	10	-.015	12	.012	1	2.642e-3	3	6770.164	12	NC	6
576		min	-.006	4	-.052	4	-.015	5	-2.456e-3	2	2020.526	4	5521.55	1
577	4	max	0	10	-.021	12	.018	1	2.529e-3	3	4644.728	12	NC	10
578		min	-.005	4	-.074	4	-.03	5	-2.339e-3	2	1386.199	4	3422.579	5
579	5	max	0	10	-.027	12	.022	1	2.416e-3	3	3624.328	12	9770.565	10
580		min	-.005	4	-.093	4	-.047	5	-2.222e-3	2	1081.665	4	2122.354	5
581	6	max	0	10	-.032	12	.024	1	2.303e-3	3	3050.255	12	9127.204	10
582		min	-.005	4	-.11	4	-.065	5	-2.106e-3	2	910.336	4	1528.626	5
583	7	max	0	10	-.036	12	.025	1	2.189e-3	3	2705.026	12	8998.254	10
584		min	-.004	4	-.123	4	-.081	5	-1.989e-3	2	807.303	4	1213.602	5
585	8	max	0	10	-.039	12	.025	1	2.076e-3	3	2497.837	12	9266.903	10
586		min	-.004	4	-.133	4	-.095	5	-1.872e-3	2	745.469	4	1034.503	5
587	9	max	0	10	-.041	12	.024	1	1.963e-3	3	2386.315	12	9925.585	10
588		min	-.003	4	-.138	4	-.105	5	-1.756e-3	2	712.185	4	933.332	5
589	10	max	0	10	-.041	12	.021	1	1.85e-3	3	2351.035	12	NC	10
590		min	-.003	4	-.14	4	-.11	5	-1.639e-3	2	701.656	4	884.273	5
591	11	max	0	10	-.041	12	.019	1	1.736e-3	3	2386.315	12	NC	10
592		min	-.003	4	-.138	4	-.111	5	-1.522e-3	2	712.185	4	876.615	5
593	12	max	0	10	-.039	12	.015	1	1.623e-3	3	2497.837	12	NC	9
594		min	-.002	4	-.132	4	-.107	5	-1.406e-3	2	745.469	4	909.015	5
595	13	max	0	10	-.036	12	.012	1	1.51e-3	3	2705.026	12	NC	3
596		min	-.002	4	-.121	4	-.099	5	-1.289e-3	2	807.303	4	988.961	5
597	14	max	0	10	-.032	12	.009	1	1.397e-3	3	3050.255	12	NC	2
598		min	-.002	4	-.108	4	-.086	5	-1.172e-3	2	910.336	4	1137.037	5
599	15	max	0	10	-.027	12	.006	1	1.283e-3	3	3624.328	12	NC	1
600		min	-.001	4	-.091	4	-.07	5	-1.056e-3	2	1081.665	4	1400.95	5
601	16	max	0	10	-.021	12	.003	1	1.17e-3	3	4644.728	12	NC	1
602		min	-.001	4	-.071	4	-.051	5	-9.392e-4	2	1386.199	4	1900.639	5
603	17	max	0	10	-.014	12	.001	9	1.057e-3	3	6770.164	12	NC	1
604		min	0	4	-.048	4	-.032	5	-8.226e-4	2	2020.526	4	3014.375	5
605	18	max	0	10	-.007	12	0	3	1.108e-3	4	NC	12	NC	1
606		min	0	4	-.025	4	-.015	5	-7.059e-4	2	3970.646	4	6706.204	5
607	19	max	0	1	0	1	0	1	1.182e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.893e-4	2	NC	1	NC	1



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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