

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

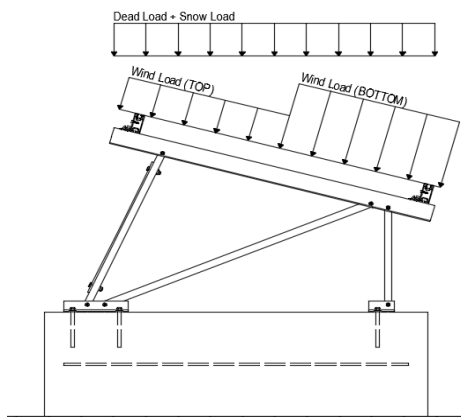
1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

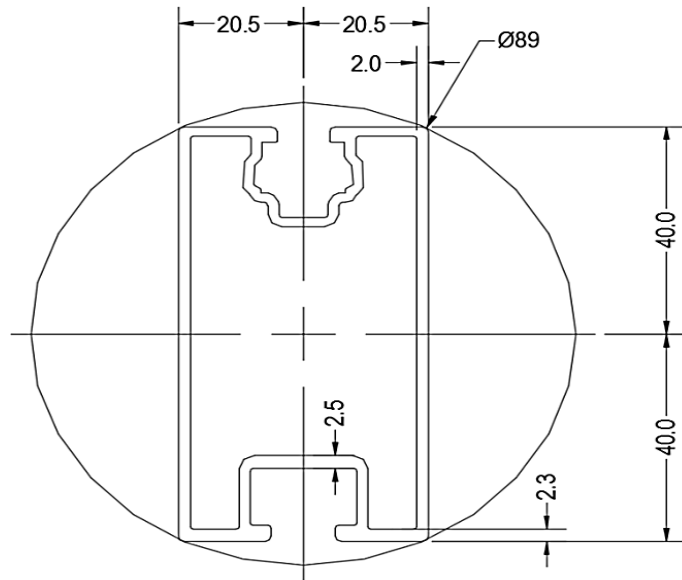
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	93 in
ΦF_{ty} STRONG-AXIS =	28.83 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.058 k-ft
M_z =	0.197 k-ft
$M_{y \text{ allowable}}$ =	1.791 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	83%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

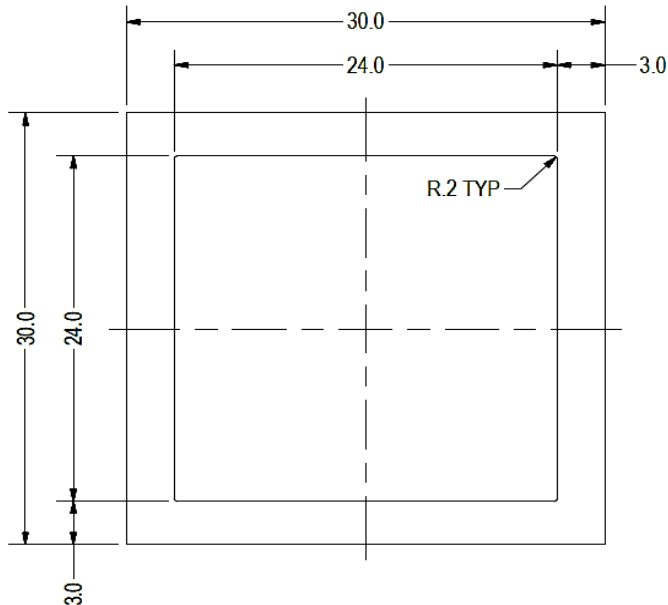
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.72 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.565 k-ft
M_z =	0.000 k-ft
P_n =	0.226 k
$M_{y \text{ allowable}}$ =	1.458 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	41%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.053 k-ft
P_n =	0.293 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	15%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.322 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.220 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.007 k-ft
P_n =	0.255 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	18%



A cross brace kit is required every 11 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

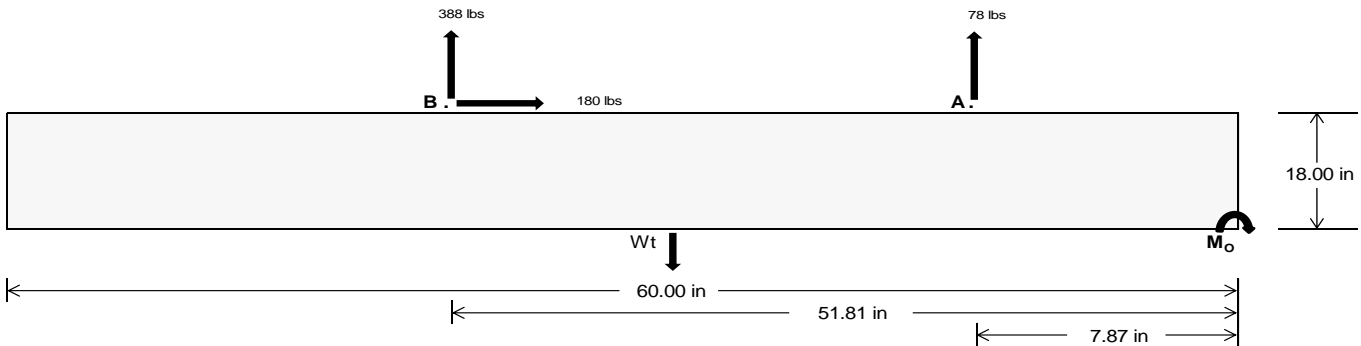
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	330.39	1617.06	k
Compressive Load =	1953.20	1514.63	k
Lateral Load =	42.87	749.41	k
Moment (Weak Axis) =	0.07	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23949.2$ in-lbs
Resisting Force Required = 798.31 lbs
S.F. = 1.67
Weight Required = 1330.51 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 180.07 lbs
Friction = 0.4
Weight Required = 450.19 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 180.07 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	740 lbs	740 lbs	740 lbs	740 lbs	564 lbs	564 lbs	564 lbs	564 lbs	923 lbs	923 lbs	923 lbs	923 lbs	-156 lbs	-156 lbs	-156 lbs	-156 lbs
F_B	541 lbs	541 lbs	541 lbs	541 lbs	497 lbs	497 lbs	497 lbs	497 lbs	737 lbs	737 lbs	737 lbs	737 lbs	-776 lbs	-776 lbs	-776 lbs	-776 lbs
F_V	65 lbs	65 lbs	65 lbs	65 lbs	324 lbs	324 lbs	324 lbs	324 lbs	288 lbs	288 lbs	288 lbs	288 lbs	-360 lbs	-360 lbs	-360 lbs	-360 lbs
P_{total}	3275 lbs	3366 lbs	3456 lbs	3547 lbs	3055 lbs	3146 lbs	3236 lbs	3327 lbs	3654 lbs	3745 lbs	3835 lbs	3926 lbs	264 lbs	319 lbs	373 lbs	428 lbs
M	479 lbs-ft	479 lbs-ft	479 lbs-ft	479 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	793 lbs-ft	793 lbs-ft	793 lbs-ft	793 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.22 ft	0.21 ft	0.21 ft	0.21 ft	0.20 ft	2.20 ft	1.82 ft	1.56 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	294.6 psf	291.3 psf	288.2 psf	285.4 psf	251.8 psf	250.3 psf	249.0 psf	247.7 psf	294.8 psf	291.5 psf	288.4 psf	285.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	419.9 psf	411.1 psf	403.1 psf	395.6 psf	414.7 psf	406.2 psf	398.3 psf	391.1 psf	502.4 psf	490.1 psf	478.7 psf	468.3 psf	319.7 psf	164.1 psf	132.1 psf	120.0 psf

Maximum Bearing Pressure = 502 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

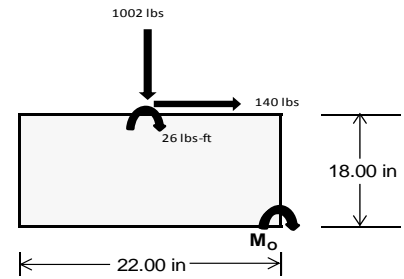
Overturning Check

$M_o = 682.3 \text{ ft-lbs}$
 Resisting Force Required = 744.31 lbs
 S.F. = 1.67
 Weight Required = 1240.52 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	139 lbs	184 lbs	86 lbs	383 lbs	1002 lbs	341 lbs	77 lbs	15 lbs	28 lbs
F_v	23 lbs	185 lbs	23 lbs	15 lbs	140 lbs	18 lbs	23 lbs	185 lbs	23 lbs
P_{total}	2607 lbs	2652 lbs	2554 lbs	2732 lbs	3352 lbs	2691 lbs	799 lbs	736 lbs	749 lbs
M	65 lbs-ft	314 lbs-ft	70 lbs-ft	43 lbs-ft	236 lbs-ft	55 lbs-ft	67 lbs-ft	312 lbs-ft	70 lbs-ft
e	0.02 ft	0.12 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.08 ft	0.42 ft	0.09 ft
$L/6$	0.31 ft	1.60 ft	1.78 ft	1.80 ft	1.69 ft	1.79 ft	1.66 ft	0.99 ft	1.65 ft
f_{min}	261.2 sqft	177.4 sqft	253.6 sqft	282.9 sqft	281.3 sqft	273.9 sqft	63.1 sqft	-31.1 sqft	56.8 sqft
f_{max}	307.6 psf	401.3 psf	303.6 psf	313.2 psf	450.0 psf	313.2 psf	111.2 psf	191.8 psf	106.7 psf



Maximum Bearing Pressure = 450 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

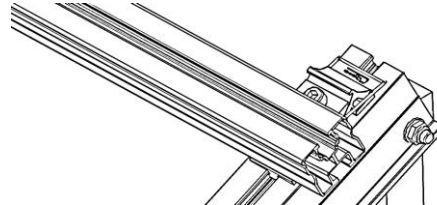
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.399 k
Allowable Uplift =	1.214 k
Utilization =	<u>33%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.085 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.502 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>26%</u>

Diagonal Strut

Maximum Axial Load =	0.322 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.220 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.255 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.111 in
	<u>0.111 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$193.965$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$210.771$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.791 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.29 \\
 &21.6567 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.29 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.458 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

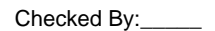
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 20° 90mph 30psf 7.75ft 7-05.rdb Page 20





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	373.897	1	.072	2	.728	1	0	10	.001	1	0	15
30			min	-358.078	3	.014	15	-.372	5	-.001	1	0	3	0	6
31		16	max	374.004	1	.04	2	.728	1	0	10	.002	1	0	15
32			min	-357.998	3	-.005	3	-.469	5	-.001	1	0	3	0	6
33		17	max	374.11	1	.01	10	.728	1	0	10	.002	1	0	15
34			min	-357.918	3	-.03	1	-.565	5	-.001	1	0	3	0	6
35		18	max	374.217	1	-.015	15	.728	1	0	10	.002	1	0	15
36			min	-357.838	3	-.062	1	-.662	5	-.001	1	0	3	0	6
37		19	max	374.323	1	-.025	15	.728	1	0	10	.002	1	0	15
38			min	-357.758	3	-.103	4	-.758	5	-.001	1	0	3	0	6
39	M3	1	max	63.878	2	1.792	6	-.033	12	0	5	.002	1	0	6
40			min	-98.261	9	.421	15	-1.485	4	0	1	0	12	0	15
41		2	max	63.81	2	1.615	6	-.033	12	0	5	.002	1	0	6
42			min	-98.317	9	.379	15	-1.352	4	0	1	0	12	0	15
43		3	max	63.742	2	1.437	6	-.033	12	0	5	.002	1	0	2
44			min	-98.374	9	.337	15	-1.218	4	0	1	0	12	0	3
45		4	max	63.674	2	1.26	6	-.033	12	0	5	.002	1	0	15
46			min	-98.43	9	.296	15	-1.085	4	0	1	0	5	0	4
47		5	max	63.606	2	1.082	6	-.033	12	0	5	.001	1	0	15
48			min	-98.487	9	.254	15	-.951	4	0	1	0	5	0	4
49		6	max	63.538	2	.904	6	-.033	12	0	5	.001	1	0	15
50			min	-98.543	9	.212	15	-.817	4	0	1	0	5	0	4
51		7	max	63.47	2	.727	6	-.033	12	0	5	.001	1	0	15
52			min	-98.6	9	.17	15	-.688	1	0	1	0	5	0	4
53		8	max	63.403	2	.549	6	-.033	12	0	5	.001	1	0	15
54			min	-98.657	9	.128	15	-.688	1	0	1	0	5	-.001	4
55		9	max	63.335	2	.371	6	-.033	12	0	5	0	1	0	15
56			min	-98.713	9	.087	15	-.688	1	0	1	0	5	-.001	4
57		10	max	63.267	2	.194	6	-.033	12	0	5	0	1	0	15
58			min	-98.77	9	.045	15	-.688	1	0	1	0	5	-.001	4
59		11	max	63.199	2	.03	2	-.007	15	0	5	0	1	0	15
60			min	-98.826	9	-.003	3	-.688	1	0	1	0	5	-.001	4
61		12	max	63.131	2	-.039	15	.122	5	0	5	0	1	0	15
62			min	-98.883	9	-.162	4	-.688	1	0	1	0	5	-.001	4
63		13	max	63.063	2	-.08	15	.255	5	0	5	0	1	0	15
64			min	-98.939	9	-.339	4	-.688	1	0	1	0	5	-.001	4
65		14	max	62.995	2	-.122	15	.389	5	0	5	0	1	0	15
66			min	-98.996	9	-.517	4	-.688	1	0	1	0	5	-.001	4
67		15	max	62.927	2	-.164	15	.523	5	0	5	0	1	0	15
68			min	-99.052	9	-.695	4	-.688	1	0	1	0	5	0	4
69		16	max	62.86	2	-.206	15	.656	5	0	5	0	12	0	15
70			min	-99.109	9	-.872	4	-.688	1	0	1	0	4	0	4
71		17	max	62.792	2	-.247	15	.79	5	0	5	0	12	0	15
72			min	-99.166	9	-1.05	4	-.688	1	0	1	0	4	0	4
73		18	max	62.724	2	-.289	15	.923	5	0	5	0	12	0	15
74			min	-99.222	9	-1.228	4	-.688	1	0	1	0	1	0	4
75		19	max	62.656	2	-.331	15	1.057	5	0	5	0	5	0	1
76			min	-99.279	9	-1.405	4	-.688	1	0	1	0	1	0	1
77	M4	1	max	524.188	1	0	1	-.138	12	0	1	0	5	0	1
78			min	-69.751	3	0	1	-32.236	4	0	1	0	1	0	1
79		2	max	524.252	1	0	1	-.138	12	0	1	0	12	0	1
80			min	-69.702	3	0	1	-32.292	4	0	1	-.003	4	0	1
81		3	max	524.317	1	0	1	-.138	12	0	1	0	12	0	1
82			min	-69.654	3	0	1	-32.348	4	0	1	-.006	4	0	1
83		4	max	524.382	1	0	1	-.138	12	0	1	0	12	0	1
84			min	-69.605	3	0	1	-32.404	4	0	1	-.009	4	0	1
85		5	max	524.446	1	0	1	-.138	12	0	1	0	12	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-69.557	3	0	1	-32.46	4	0	1	-.012	4	0	1
87	6	max	524.511	1	0	1	-.138	12	0	1	0	12	0	1
88		min	-69.508	3	0	1	-32.516	4	0	1	-.014	4	0	1
89	7	max	524.576	1	0	1	-.138	12	0	1	0	12	0	1
90		min	-69.46	3	0	1	-32.572	4	0	1	-.017	4	0	1
91	8	max	524.641	1	0	1	-.138	12	0	1	0	12	0	1
92		min	-69.411	3	0	1	-32.628	4	0	1	-.02	4	0	1
93	9	max	524.705	1	0	1	-.138	12	0	1	0	12	0	1
94		min	-69.363	3	0	1	-32.684	4	0	1	-.023	4	0	1
95	10	max	524.77	1	0	1	-.138	12	0	1	0	12	0	1
96		min	-69.314	3	0	1	-32.74	4	0	1	-.026	4	0	1
97	11	max	524.835	1	0	1	-.138	12	0	1	0	12	0	1
98		min	-69.266	3	0	1	-32.796	4	0	1	-.029	4	0	1
99	12	max	524.899	1	0	1	-.138	12	0	1	0	12	0	1
100		min	-69.217	3	0	1	-32.852	4	0	1	-.032	4	0	1
101	13	max	524.964	1	0	1	-.138	12	0	1	0	12	0	1
102		min	-69.169	3	0	1	-32.909	4	0	1	-.035	4	0	1
103	14	max	525.029	1	0	1	-.138	12	0	1	0	12	0	1
104		min	-69.12	3	0	1	-32.965	4	0	1	-.038	4	0	1
105	15	max	525.094	1	0	1	-.138	12	0	1	0	12	0	1
106		min	-69.071	3	0	1	-33.021	4	0	1	-.041	4	0	1
107	16	max	525.158	1	0	1	-.138	12	0	1	0	12	0	1
108		min	-69.023	3	0	1	-33.077	4	0	1	-.044	4	0	1
109	17	max	525.223	1	0	1	-.138	12	0	1	0	12	0	1
110		min	-68.974	3	0	1	-33.133	4	0	1	-.047	4	0	1
111	18	max	525.288	1	0	1	-.138	12	0	1	0	12	0	1
112		min	-68.926	3	0	1	-33.189	4	0	1	-.05	4	0	1
113	19	max	525.352	1	0	1	-.138	12	0	1	0	12	0	1
114		min	-68.877	3	0	1	-33.245	4	0	1	-.053	4	0	1
115	M6	1	max	1217.896	1	.627	6	1.079	4	0	0	4	0	1
116		min	-1172.656	3	.144	15	-.125	3	0	5	0	1	0	1
117	2	max	1218.002	1	.585	6	.982	4	0	1	0	4	0	15
118		min	-1172.576	3	.134	15	-.125	3	0	5	0	11	0	6
119	3	max	1218.109	1	.544	6	.886	4	0	1	0	4	0	15
120		min	-1172.496	3	.124	15	-.125	3	0	5	0	10	0	6
121	4	max	1218.215	1	.503	6	.789	4	0	1	0	4	0	15
122		min	-1172.416	3	.114	15	-.125	3	0	5	0	12	0	6
123	5	max	1218.322	1	.462	6	.693	4	0	1	0	4	0	15
124		min	-1172.336	3	.105	15	-.125	3	0	5	0	3	0	6
125	6	max	1218.428	1	.42	6	.597	4	0	1	0	4	0	15
126		min	-1172.256	3	.095	15	-.125	3	0	5	0	3	0	6
127	7	max	1218.535	1	.379	6	.5	4	0	1	0	4	0	15
128		min	-1172.176	3	.085	15	-.125	3	0	5	0	3	0	6
129	8	max	1218.641	1	.346	2	.404	4	0	1	0	4	0	15
130		min	-1172.096	3	.076	15	-.125	3	0	5	0	3	0	6
131	9	max	1218.748	1	.314	2	.313	14	0	1	0	4	0	15
132		min	-1172.016	3	.066	15	-.125	3	0	5	0	3	0	6
133	10	max	1218.855	1	.282	2	.264	14	0	1	0	4	0	15
134		min	-1171.936	3	.056	15	-.125	3	0	5	0	3	0	6
135	11	max	1218.961	1	.25	2	.263	1	0	1	0	4	0	15
136		min	-1171.857	3	.047	15	-.125	3	0	5	0	3	0	6
137	12	max	1219.068	1	.218	2	.263	1	0	1	0	4	0	15
138		min	-1171.777	3	.035	12	-.125	3	0	5	0	3	0	6
139	13	max	1219.174	1	.185	2	.263	1	0	1	0	4	0	15
140		min	-1171.697	3	.019	12	-.166	5	0	5	0	3	0	6
141	14	max	1219.281	1	.153	2	.263	1	0	1	0	4	0	15
142		min	-1171.617	3	0	3	-.262	5	0	5	0	3	0	2





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-254.825	3	0	1	-32.692	4	0	1	-.012	4	0	1
201		6	max	1501.618	1	0	1	.767	1	0	1	0	1	0	1
202			min	-254.776	3	0	1	-32.748	4	0	1	-.015	4	0	1
203		7	max	1501.683	1	0	1	.767	1	0	1	0	1	0	1
204			min	-254.728	3	0	1	-32.804	4	0	1	-.017	4	0	1
205		8	max	1501.748	1	0	1	.767	1	0	1	0	1	0	1
206			min	-254.679	3	0	1	-32.86	4	0	1	-.02	4	0	1
207		9	max	1501.812	1	0	1	.767	1	0	1	0	1	0	1
208			min	-254.631	3	0	1	-32.916	4	0	1	-.023	4	0	1
209		10	max	1501.877	1	0	1	.767	1	0	1	0	1	0	1
210			min	-254.582	3	0	1	-32.972	4	0	1	-.026	4	0	1
211		11	max	1501.942	1	0	1	.767	1	0	1	0	1	0	1
212			min	-254.534	3	0	1	-33.028	4	0	1	-.029	4	0	1
213		12	max	1502.006	1	0	1	.767	1	0	1	0	1	0	1
214			min	-254.485	3	0	1	-33.084	4	0	1	-.032	4	0	1
215		13	max	1502.071	1	0	1	.767	1	0	1	0	1	0	1
216			min	-254.437	3	0	1	-33.14	4	0	1	-.035	4	0	1
217		14	max	1502.136	1	0	1	.767	1	0	1	0	1	0	1
218			min	-254.388	3	0	1	-33.196	4	0	1	-.038	4	0	1
219		15	max	1502.201	1	0	1	.767	1	0	1	0	1	0	1
220			min	-254.339	3	0	1	-33.252	4	0	1	-.041	4	0	1
221		16	max	1502.265	1	0	1	.767	1	0	1	.001	1	0	1
222			min	-254.291	3	0	1	-33.308	4	0	1	-.044	4	0	1
223		17	max	1502.33	1	0	1	.767	1	0	1	.001	1	0	1
224			min	-254.242	3	0	1	-33.365	4	0	1	-.047	4	0	1
225		18	max	1502.395	1	0	1	.767	1	0	1	.001	1	0	1
226			min	-254.194	3	0	1	-33.421	4	0	1	-.05	4	0	1
227		19	max	1502.459	1	0	1	.767	1	0	1	.001	1	0	1
228			min	-254.145	3	0	1	-33.477	4	0	1	-.053	4	0	1
229	M10	1	max	383.883	1	.666	4	1.281	5	.001	1	0	1	0	1
230			min	-346.015	3	.167	15	-.152	1	-.002	5	0	3	0	1
231		2	max	383.99	1	.625	4	1.184	5	.001	1	0	4	0	15
232			min	-345.935	3	.158	15	-.152	1	-.002	5	0	3	0	4
233		3	max	384.096	1	.583	4	1.088	5	.001	1	0	4	0	15
234			min	-345.855	3	.148	15	-.152	1	-.002	5	0	3	0	4
235		4	max	384.203	1	.542	4	.991	5	.001	1	0	4	0	15
236			min	-345.775	3	.138	15	-.152	1	-.002	5	0	3	0	4
237		5	max	384.309	1	.501	4	.895	5	.001	1	0	4	0	15
238			min	-345.695	3	.128	15	-.152	1	-.002	5	0	3	0	4
239		6	max	384.416	1	.459	4	.799	5	.001	1	0	4	0	15
240			min	-345.615	3	.119	15	-.152	1	-.002	5	0	3	0	4
241		7	max	384.522	1	.418	4	.702	5	.001	1	0	4	0	15
242			min	-345.535	3	.109	15	-.152	1	-.002	5	0	3	0	4
243		8	max	384.629	1	.377	4	.606	5	.001	1	.001	4	0	15
244			min	-345.455	3	.099	15	-.152	1	-.002	5	0	3	0	4
245		9	max	384.736	1	.336	4	.509	5	.001	1	.001	4	0	15
246			min	-345.376	3	.09	15	-.152	1	-.002	5	0	3	0	4
247		10	max	384.842	1	.294	4	.413	5	.001	1	.001	4	0	15
248			min	-345.296	3	.08	15	-.152	1	-.002	5	0	3	0	4
249		11	max	384.949	1	.253	4	.316	5	.001	1	.001	4	0	15
250			min	-345.216	3	.07	15	-.152	1	-.002	5	0	1	0	4
251		12	max	385.055	1	.212	4	.22	5	.001	1	.001	4	0	15
252			min	-345.136	3	.061	15	-.152	1	-.002	5	0	1	0	4
253		13	max	385.162	1	.171	4	.123	5	.001	1	.001	4	0	15
254			min	-345.056	3	.051	15	-.152	1	-.002	5	0	1	0	4
255		14	max	385.268	1	.129	4	.027	5	.001	1	.001	4	0	15
256			min	-344.976	3	.019	1	-.152	1	-.002	5	0	1	0	4







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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	111.41	1	4.401	9	-2.957	12	0	15	-.003	12	.181	2
372			min	6.18	10	-23.366	3	-71.996	1	0	1	-.077	1	-.098	3
373		16	max	81.096	2	29.892	10	-2.99	12	0	1	-.003	12	.184	2
374			min	-30.665	3	-86.441	3	-72.598	1	0	5	-.094	1	-.092	3
375		17	max	81.191	2	29.673	10	-2.99	12	0	1	-.004	12	.196	1
376			min	-30.594	3	-86.638	3	-72.598	1	0	5	-.109	1	-.074	3
377		18	max	-4.448	12	421.168	1	-3.13	12	0	5	-.005	12	.106	1
378			min	-130.057	1	-152.447	3	-74.347	1	0	1	-.126	1	-.041	3
379		19	max	-4.4	12	420.906	1	-3.13	12	0	5	-.005	12	.015	1
380			min	-129.961	1	-152.643	3	-74.347	1	0	1	-.142	1	-.008	3
381	M5	1	max	287.94	1	1115.344	3	-.065	10	0	1	.047	4	.021	3
382			min	6.485	15	-1228.907	1	-29.239	3	0	5	0	10	-.028	1
383		2	max	288.036	1	1115.147	3	-.065	10	0	1	.041	4	.239	1
384			min	6.514	15	-1229.169	1	-29.239	3	0	5	-.003	3	-.221	3
385		3	max	220.999	1	8.766	9	3.324	3	0	3	.034	4	.501	1
386			min	5.069	15	-69.227	3	-25.505	4	0	4	-.009	3	-.458	3
387		4	max	221.095	1	8.547	9	3.324	3	0	3	.029	4	.505	1
388			min	5.098	15	-69.423	3	-25.263	4	0	4	-.008	3	-.443	3
389		5	max	221.19	1	8.328	9	3.324	3	0	3	.023	4	.509	1
390			min	5.127	15	-69.62	3	-25.021	4	0	4	-.008	3	-.427	3
391		6	max	221.286	1	8.11	9	3.324	3	0	3	.018	4	.514	1
392			min	5.156	15	-69.817	3	-24.779	4	0	4	-.007	3	-.412	3
393		7	max	221.381	1	7.891	9	3.324	3	0	3	.013	4	.518	1
394			min	5.185	15	-70.014	3	-24.537	4	0	4	-.006	3	-.397	3
395		8	max	221.477	1	7.672	9	3.324	3	0	3	.007	4	.523	1
396			min	5.213	15	-70.211	3	-24.295	4	0	4	-.005	3	-.382	3
397		9	max	221.572	1	7.454	9	3.324	3	0	3	.002	5	.527	1
398			min	5.242	15	-70.407	3	-24.053	4	0	4	-.005	3	-.367	3
399		10	max	221.668	1	7.235	9	3.324	3	0	3	0	10	.532	1
400			min	5.271	15	-70.604	3	-23.811	4	0	4	-.004	3	-.351	3
401		11	max	221.763	1	7.016	9	3.324	3	0	3	0	10	.537	1
402			min	5.3	15	-70.801	3	-23.569	4	0	4	-.008	4	-.336	3
403		12	max	221.859	1	6.798	9	3.324	3	0	3	0	10	.542	1
404			min	5.329	15	-70.998	3	-23.327	4	0	4	-.013	4	-.321	3
405		13	max	221.954	1	6.579	9	3.324	3	0	3	0	10	.547	1
406			min	5.357	15	-71.195	3	-23.085	4	0	4	-.018	4	-.305	3
407		14	max	222.05	1	6.36	9	3.324	3	0	3	0	10	.552	1
408			min	5.386	15	-71.391	3	-22.843	4	0	4	-.023	4	-.29	3
409		15	max	222.145	1	6.142	9	3.324	3	0	3	0	10	.556	1
410			min	5.415	15	-71.588	3	-22.601	4	0	4	-.028	4	-.274	3
411		16	max	289.083	2	170.657	2	3.299	3	0	1	0	3	.561	1
412			min	-100.473	3	-259.717	3	-21.406	4	0	4	-.033	4	-.257	3
413		17	max	289.178	2	170.394	2	3.299	3	0	1	0	3	.565	1
414			min	-100.402	3	-259.913	3	-21.164	4	0	4	-.038	4	-.201	3
415		18	max	-8.927	12	1384.719	1	3.034	3	0	4	.002	3	.271	1
416			min	-288.723	1	-500.457	3	-52.133	5	0	1	-.049	4	-.093	3
417		19	max	-8.879	12	1384.457	1	3.034	3	0	4	.002	3	.015	3
418			min	-288.628	1	-500.654	3	-51.891	5	0	1	-.06	4	-.03	1
419	M9	1	max	129.861	1	338.014	3	216.348	4	0	3	-.001	15	.014	1
420			min	2.401	15	-372.296	1	6.076	10	0	1	-.142	1	-.011	3
421		2	max	129.956	1	337.817	3	216.59	4	0	3	.042	5	.095	1
422			min	2.43	15	-372.558	1	6.076	10	0	1	-.122	1	-.084	3
423		3	max	110.236	1	6.999	9	68.022	1	0	1	.083	5	.174	1
424			min	2.244	15	-20.948	3	-33.191	5	0	12	-.1	1	-.156	3
425		4	max	110.332	1	6.78	9	68.022	1	0	1	.075	5	.174	1
426			min	2.273	15	-21.145	3	-32.949	5	0	12	-.085	1	-.151	3
427		5	max	110.427	1	6.561	9	68.022	1	0	1	.068	5	.174	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	2.302	15	-21.342	3	-32.707	5	0	12	-.07	1	-.146	3
429		6	max	110.523	1	6.343	9	68.022	1	0	1	.061	5	.174	1
430			min	2.331	15	-21.538	3	-32.465	5	0	12	-.056	1	-.142	3
431		7	max	110.618	1	6.124	9	68.022	1	0	1	.054	5	.175	1
432			min	2.36	15	-21.735	3	-32.223	5	0	12	-.041	1	-.137	3
433		8	max	110.714	1	5.905	9	68.022	1	0	1	.047	5	.175	1
434			min	2.388	15	-21.932	3	-31.981	5	0	12	-.026	1	-.132	3
435		9	max	110.809	1	5.687	9	68.022	1	0	1	.04	5	.175	1
436			min	2.417	15	-22.129	3	-31.739	5	0	12	-.011	1	-.128	3
437		10	max	110.905	1	5.468	9	68.022	1	0	1	.034	4	.176	1
438			min	2.446	15	-22.326	3	-31.497	5	0	12	0	2	-.123	3
439		11	max	111.001	1	5.249	9	68.022	1	0	1	.03	4	.176	1
440			min	2.475	15	-22.522	3	-31.255	5	0	12	.001	10	-.118	3
441		12	max	111.096	1	5.031	9	68.022	1	0	1	.033	1	.177	1
442			min	2.504	15	-22.719	3	-31.013	5	0	12	.003	10	-.113	3
443		13	max	111.192	1	4.812	9	68.022	1	0	1	.048	1	.178	1
444			min	2.533	15	-22.916	3	-30.771	5	0	12	.004	12	-.108	3
445		14	max	111.287	1	4.593	9	68.022	1	0	1	.062	1	.178	1
446			min	2.561	15	-23.113	3	-30.529	5	0	12	.004	12	-.103	3
447		15	max	111.383	1	4.375	9	68.022	1	0	1	.077	1	.181	2
448			min	2.59	15	-23.31	3	-30.287	5	0	12	0	15	-.098	3
449		16	max	81.348	2	29.538	10	68.758	1	0	10	.093	1	.184	2
450			min	-30.755	3	-86.846	3	-28.813	5	0	4	-.004	5	-.092	3
451		17	max	81.444	2	29.32	10	68.758	1	0	10	.108	1	.196	1
452			min	-30.683	3	-87.043	3	-28.571	5	0	4	-.011	5	-.074	3
453		18	max	2.976	5	421.168	1	72.436	1	0	1	.124	1	.106	1
454			min	-129.775	1	-152.445	3	-57.894	5	0	3	-.023	5	-.041	3
455		19	max	3.02	5	420.906	1	72.436	1	0	1	.14	1	.015	1
456			min	-129.679	1	-152.642	3	-57.652	5	0	3	-.036	5	-.008	3
457	M13	1	max	216.357	4	371.725	1	-2.401	15	.014	1	.142	1	0	1
458			min	6.077	10	-338.003	3	-129.846	1	-.011	3	.001	15	0	3
459		2	max	207.831	4	262.384	1	-1.386	15	.014	1	.043	1	.248	3
460			min	6.077	10	-238.492	3	-99.387	1	-.011	3	0	5	-.273	1
461		3	max	199.304	4	153.043	1	-.37	15	.014	1	.002	3	.411	3
462			min	6.077	10	-138.981	3	-68.927	1	-.011	3	-.029	1	-.452	1
463		4	max	190.778	4	43.702	1	.887	5	.014	1	0	12	.488	3
464			min	6.077	10	-39.471	3	-38.468	1	-.011	3	-.075	1	-.537	1
465		5	max	182.252	4	60.04	3	2.457	5	.014	1	0	15	.479	3
466			min	6.077	10	-65.638	1	-8.009	1	-.011	3	-.095	1	-.527	1
467		6	max	173.725	4	159.551	3	22.451	1	.014	1	.002	5	.384	3
468			min	6.077	10	-174.979	1	.242	12	-.011	3	-.089	1	-.424	1
469		7	max	165.199	4	259.062	3	52.91	1	.014	1	.006	5	.204	3
470			min	6.077	10	-284.32	1	1.232	12	-.011	3	-.057	1	-.226	1
471		8	max	156.672	4	358.572	3	83.37	1	.014	1	.012	4	.066	1
472			min	6.077	10	-393.661	1	2.223	12	-.011	3	0	3	-.062	3
473		9	max	148.146	4	458.083	3	113.829	1	.014	1	.087	1	.452	1
474			min	6.077	10	-503.001	1	3.213	12	-.011	3	.002	12	-.414	3
475		10	max	139.62	4	557.594	3	144.288	1	.011	2	.198	1	.932	1
476			min	6.077	10	-612.342	1	4.204	12	-.014	1	.006	12	-.851	3
477		11	max	102.113	4	503.001	1	1.658	5	.011	3	.083	1	.452	1
478			min	2.929	12	-458.083	3	-113.219	1	-.014	1	-.018	5	-.414	3
479		12	max	93.586	4	393.661	1	3.229	5	.011	3	.001	2	.066	1
480			min	2.929	12	-358.572	3	-82.76	1	-.014	1	-.016	4	-.062	3
481		13	max	85.06	4	284.32	1	4.799	5	.011	3	-.003	12	.204	3
482			min	2.929	12	-259.062	3	-52.3	1	-.014	1	-.059	1	-.226	1
483		14	max	76.534	4	174.979	1	6.369	5	.011	3	-.004	12	.384	3
484			min	2.929	12	-159.551	3	-21.841	1	-.014	1	-.091	1	-.424	1





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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-34.5	3	0	2	-.034	1	0	3	0	3	-.003	1
543		6	max	0	2	.935	1	.028	3	0	1	0	1	0	2
544			min	-34.559	3	0	2	-.034	1	0	3	0	3	-.003	1
545		7	max	0	2	.701	1	.028	3	0	1	0	3	0	2
546			min	-34.619	3	0	2	-.034	1	0	3	0	1	-.004	1
547		8	max	0	2	.467	1	.028	3	0	1	0	3	0	2
548			min	-34.679	3	0	2	-.034	1	0	3	0	1	-.004	1
549		9	max	0	2	.234	1	.028	3	0	1	0	3	0	2
550			min	-34.738	3	0	2	-.034	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.028	3	0	1	0	3	0	2
552			min	-34.798	3	0	1	-.034	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.028	3	0	1	0	3	0	2
554			min	-34.858	3	-.234	1	-.034	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.028	3	0	1	0	3	0	2
556			min	-34.917	3	-.467	1	-.034	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.028	3	0	1	0	3	0	2
558			min	-34.977	3	-.701	1	-.034	1	0	3	0	1	-.004	1
559		14	max	0	2	0	2	.028	3	0	1	0	3	0	2
560			min	-35.037	3	-.935	1	-.034	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.028	3	0	1	0	3	0	2
562			min	-35.096	3	-1.169	1	-.034	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.028	3	0	1	0	3	0	2
564			min	-35.156	3	-1.402	1	-.034	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.028	3	0	1	0	3	0	2
566			min	-35.216	3	-1.636	1	-.034	1	0	3	0	1	-.002	1
567		18	max	0	2	0	2	.028	3	0	1	0	3	0	2
568			min	-35.275	3	-1.87	1	-.034	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.028	3	0	1	0	3	0	1
570			min	-35.335	3	-2.104	1	-.034	1	0	3	0	1	0	1
571	M16A	1	max	-.792	10	3.317	4	.228	4	0	3	0	3	0	1
572			min	-253.248	4	1.05	15	-.012	3	0	1	0	4	0	1
573		2	max	-.726	10	2.948	4	.206	4	0	3	0	3	0	15
574			min	-253.348	4	.933	15	-.012	3	0	1	0	4	-.001	4
575		3	max	-.66	10	2.58	4	.184	4	0	3	0	3	0	15
576			min	-253.447	4	.817	15	-.012	3	0	1	0	4	-.003	4
577		4	max	-.594	10	2.211	4	.162	4	0	3	0	3	-.001	15
578			min	-253.546	4	.7	15	-.012	3	0	1	0	4	-.004	4
579		5	max	-.527	10	1.843	4	.14	4	0	3	0	3	-.001	15
580			min	-253.646	4	.583	15	-.012	3	0	1	0	1	-.005	4
581		6	max	-.461	10	1.474	4	.118	4	0	3	0	3	-.002	15
582			min	-253.745	4	.467	15	-.012	3	0	1	0	1	-.005	4
583		7	max	-.395	10	1.106	4	.096	4	0	3	0	5	-.002	15
584			min	-253.845	4	.35	15	-.012	3	0	1	0	1	-.006	4
585		8	max	-.328	10	.737	4	.074	4	0	3	0	5	-.002	15
586			min	-253.944	4	.233	15	-.012	3	0	1	0	1	-.006	4
587		9	max	-.262	10	.369	4	.052	4	0	3	0	5	-.002	15
588			min	-254.043	4	.117	15	-.012	3	0	1	0	1	-.007	4
589		10	max	-.196	10	0	1	.03	4	0	3	0	5	-.002	15
590			min	-254.143	4	0	1	-.012	3	0	1	0	1	-.007	4
591		11	max	-.129	10	-.117	15	.022	1	0	3	0	5	-.002	15
592			min	-254.242	4	-.369	4	-.012	3	0	1	0	1	-.007	4
593		12	max	-.063	10	-.233	15	.022	1	0	3	0	5	-.002	15
594			min	-254.342	4	-.737	4	-.018	5	0	1	0	1	-.006	4
595		13	max	.003	10	-.35	15	.022	1	0	3	0	5	-.002	15
596			min	-254.441	4	-1.106	4	-.04	5	0	1	0	3	-.006	4
597		14	max	.069	10	-.467	15	.022	1	0	3	0	4	-.002	15
598			min	-254.54	4	-1.474	4	-.062	5	0	1	0	3	-.005	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.136	10	-5.583	15	.022	1	0	3	0	4	-.001	15
600		min	-254.64	4	-1.843	4	-.084	5	0	1	0	3	-.005	4
601	16	max	.202	10	-.7	15	.022	1	0	3	0	4	-.001	15
602		min	-254.739	4	-2.211	4	-.106	5	0	1	0	3	-.004	4
603	17	max	.268	10	-.817	15	.022	1	0	3	0	1	0	15
604		min	-254.839	4	-2.58	4	-.128	5	0	1	0	3	-.003	4
605	18	max	.335	10	-.933	15	.022	1	0	3	0	1	0	15
606		min	-254.938	4	-2.948	4	-.15	5	0	1	0	5	-.001	4
607	19	max	.401	10	-1.05	15	.022	1	0	3	0	1	0	1
608		min	-255.037	4	-3.317	4	-.172	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.007	2	.014	1	1.821e-3	5	NC	3	NC	3	
2			min	-.003	3	-.006	3	-.018	5	-1.087e-3	1	4477.131	2	2424.803	1	
3			2	max	.003	1	.007	2	.013	1	1.847e-3	5	NC	3	NC	3
4				min	-.003	3	-.006	3	-.017	5	-1.042e-3	1	4854.968	2	2624.398	1
5			3	max	.003	1	.006	2	.012	1	1.873e-3	5	NC	3	NC	3
6				min	-.003	3	-.006	3	-.017	5	-9.979e-4	1	5298.992	2	2859.441	1
7			4	max	.003	1	.006	2	.011	1	1.899e-3	5	NC	3	NC	3
8				min	-.002	3	-.006	3	-.016	5	-9.535e-4	1	5824.276	2	3138.536	1
9			5	max	.002	1	.005	2	.01	1	1.925e-3	5	NC	3	NC	3
10				min	-.002	3	-.005	3	-.015	5	-9.091e-4	1	6450.659	2	3473.17	1
11			6	max	.002	1	.005	2	.009	1	1.952e-3	5	NC	1	NC	3
12				min	-.002	3	-.005	3	-.014	5	-8.647e-4	1	7204.674	2	3878.952	1
13			7	max	.002	1	.004	2	.008	1	1.978e-3	5	NC	1	NC	2
14				min	-.002	3	-.005	3	-.013	5	-8.203e-4	1	8122.47	2	4377.543	1
15			8	max	.002	1	.004	2	.007	1	2.004e-3	5	NC	1	NC	2
16				min	-.002	3	-.005	3	-.012	5	-7.759e-4	1	9254.38	2	4999.73	1
17			9	max	.002	1	.003	2	.006	1	2.03e-3	5	NC	1	NC	2
18				min	-.002	3	-.004	3	-.011	5	-7.315e-4	1	NC	1	5790.511	1
19			10	max	.002	1	.003	2	.005	1	2.056e-3	5	NC	1	NC	2
20				min	-.001	3	-.004	3	-.01	5	-6.872e-4	1	NC	1	6817.866	1
21		11	max	.001	1	.002	2	.004	1	2.083e-3	5	NC	1	NC	2	
22			min	-.001	3	-.004	3	-.009	5	-6.428e-4	1	NC	1	8188.638	1	
23		12	max	.001	1	.002	2	.003	1	2.109e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.008	5	-5.984e-4	1	NC	1	NC	1	
25		13	max	.001	1	.001	2	.003	1	2.135e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.007	5	-5.54e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.161e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.006	5	-5.096e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	2.187e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.005	5	-4.652e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	2.214e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.004	5	-4.208e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.24e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.003	5	-3.764e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.266e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-3.32e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.292e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.876e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.322e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-1.054e-3	5	NC	1	NC	1	
41			2	max	0	9	0	2	.006	5	1.663e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-1.061e-3	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	9	0	2	.011	5	2.004e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.068e-3	5	NC	1	8832.316	14
45		4	max	0	9	0	2	.017	5	2.345e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	-.001	1	-1.074e-3	5	NC	1	5769.916	14
47		5	max	0	9	0	2	.022	5	2.685e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	-1.081e-3	5	NC	1	4252.029	14
49		6	max	0	9	0	2	.028	4	3.026e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.088e-3	5	NC	1	3350.542	14
51		7	max	0	9	0	2	.034	4	3.367e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-1.095e-3	5	NC	1	2756.28	14
53		8	max	0	9	.001	2	.039	4	3.708e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-1.102e-3	5	NC	1	2336.871	14
55		9	max	0	9	.002	2	.045	4	4.049e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-1.109e-3	5	NC	1	2026.22	14
57		10	max	0	9	.002	2	.05	4	4.39e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-1.115e-3	5	NC	1	1787.669	14
59		11	max	0	9	.003	2	.056	4	4.731e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-1.122e-3	5	NC	1	1599.268	14
61		12	max	0	9	.003	2	.061	4	5.072e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	12	-1.129e-3	5	NC	1	1447.081	14
63		13	max	0	9	.004	2	.066	4	5.413e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	12	-1.136e-3	5	NC	1	1321.839	14
65		14	max	0	9	.005	2	.072	4	5.754e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	12	-1.143e-3	5	9814.131	2	1217.142	14
67		15	max	0	9	.006	2	.077	4	6.095e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	12	-1.15e-3	5	8272.569	2	1128.428	14
69		16	max	0	9	.007	2	.082	4	6.435e-4	1	NC	3	NC	2
70			min	0	2	-.007	3	0	12	-1.156e-3	5	7073.684	2	1052.36	14
71		17	max	0	9	.008	2	.087	4	6.776e-4	1	NC	3	NC	2
72			min	0	2	-.008	3	0	12	-1.163e-3	5	6132.037	2	986.439	14
73		18	max	.001	9	.009	2	.092	4	7.117e-4	1	NC	3	NC	2
74			min	0	2	-.008	3	0	12	-1.17e-3	5	5386.065	2	928.76	14
75		19	max	.001	9	.01	2	.097	4	7.458e-4	1	NC	3	NC	2
76			min	0	2	-.008	3	0	12	-1.177e-3	5	4791.126	2	877.838	14
77	M4	1	max	.002	1	.009	2	0	12	5.001e-3	5	NC	1	NC	2
78			min	0	3	-.007	3	-.103	4	-9.086e-4	1	NC	1	187.768	4
79		2	max	.002	1	.008	2	0	12	5.001e-3	5	NC	1	NC	2
80			min	0	3	-.006	3	-.094	4	-9.086e-4	1	NC	1	204.695	4
81		3	max	.002	1	.008	2	0	12	5.001e-3	5	NC	1	NC	2
82			min	0	3	-.006	3	-.086	4	-9.086e-4	1	NC	1	224.843	4
83		4	max	.002	1	.007	2	0	12	5.001e-3	5	NC	1	NC	2
84			min	0	3	-.006	3	-.078	4	-9.086e-4	1	NC	1	249.059	4
85		5	max	.002	1	.007	2	0	12	5.001e-3	5	NC	1	NC	2
86			min	0	3	-.005	3	-.069	4	-9.086e-4	1	NC	1	278.501	4
87		6	max	.002	1	.006	2	0	12	5.001e-3	5	NC	1	NC	2
88			min	0	3	-.005	3	-.061	4	-9.086e-4	1	NC	1	314.774	4
89		7	max	.002	1	.006	2	0	12	5.001e-3	5	NC	1	NC	2
90			min	0	3	-.004	3	-.054	4	-9.086e-4	1	NC	1	360.168	4
91		8	max	.002	1	.005	2	0	12	5.001e-3	5	NC	1	NC	2
92			min	0	3	-.004	3	-.046	4	-9.086e-4	1	NC	1	418.034	4
93		9	max	.001	1	.005	2	0	12	5.001e-3	5	NC	1	NC	2
94			min	0	3	-.004	3	-.039	4	-9.086e-4	1	NC	1	493.443	4
95		10	max	.001	1	.004	2	0	12	5.001e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.033	4	-9.086e-4	1	NC	1	594.363	4
97		11	max	.001	1	.004	2	0	12	5.001e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.026	4	-9.086e-4	1	NC	1	733.92	4
99		12	max	0	1	.003	2	0	12	5.001e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.003	3	-.021	4	-9.086e-4	1	NC	1	935.008	4
101		13	max	0	1	.003	2	0	12	5.001e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.016	4	-9.086e-4	1	NC	1	1240.575	4
103		14	max	0	1	.002	2	0	12	5.001e-3	5	NC	1	NC	1
104			min	0	3	-.002	3	-.011	4	-9.086e-4	1	NC	1	1739.203	4
105		15	max	0	1	.002	2	0	12	5.001e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.007	4	-9.086e-4	1	NC	1	2639.014	4
107		16	max	0	1	.001	2	0	12	5.001e-3	5	NC	1	NC	1
108			min	0	3	-.001	3	-.004	4	-9.086e-4	1	NC	1	4531.65	4
109		17	max	0	1	0	2	0	12	5.001e-3	5	NC	1	NC	1
110			min	0	3	0	3	-.002	4	-9.086e-4	1	NC	1	9714.944	4
111		18	max	0	1	0	2	0	12	5.001e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-9.086e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	5.001e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.086e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.024	2	.004	1	2.005e-3	4	NC	3	NC	2
116			min	-.01	3	-.018	3	-.018	5	2.485e-6	10	1395.851	2	7815.263	1
117		2	max	.009	1	.022	2	.004	1	2.027e-3	4	NC	3	NC	2
118			min	-.009	3	-.017	3	-.017	5	1.781e-6	10	1490.338	2	8474.696	1
119		3	max	.009	1	.021	2	.004	1	2.049e-3	4	NC	3	NC	2
120			min	-.009	3	-.016	3	-.017	5	1.078e-6	10	1598.199	2	9256.4	1
121		4	max	.008	1	.019	2	.003	1	2.071e-3	4	NC	3	NC	1
122			min	-.008	3	-.015	3	-.016	5	3.74e-7	10	1722.109	2	NC	1
123		5	max	.008	1	.018	2	.003	1	2.093e-3	4	NC	3	NC	1
124			min	-.007	3	-.014	3	-.015	5	-3.297e-7	10	1865.511	2	NC	1
125		6	max	.007	1	.016	2	.003	1	2.115e-3	4	NC	3	NC	1
126			min	-.007	3	-.013	3	-.014	5	-1.033e-6	10	2032.914	2	NC	1
127		7	max	.007	1	.015	2	.002	1	2.137e-3	4	NC	3	NC	1
128			min	-.006	3	-.013	3	-.013	5	-3.452e-6	2	2230.331	2	NC	1
129		8	max	.006	1	.013	2	.002	1	2.159e-3	4	NC	3	NC	1
130			min	-.006	3	-.012	3	-.013	5	-7.098e-6	2	2465.968	2	NC	1
131		9	max	.006	1	.012	2	.002	1	2.181e-3	4	NC	3	NC	1
132			min	-.005	3	-.011	3	-.012	5	-1.074e-5	2	2751.315	2	NC	1
133		10	max	.005	1	.011	2	.001	1	2.203e-3	4	NC	3	NC	1
134			min	-.005	3	-.01	3	-.011	5	-1.439e-5	2	3102.974	2	NC	1
135		11	max	.004	1	.009	2	.001	1	2.225e-3	4	NC	3	NC	1
136			min	-.004	3	-.009	3	-.01	5	-1.804e-5	2	3545.845	2	NC	1
137		12	max	.004	1	.008	2	0	1	2.248e-3	4	NC	3	NC	1
138			min	-.004	3	-.008	3	-.009	5	-2.168e-5	2	4119.06	2	NC	1
139		13	max	.003	1	.007	2	0	1	2.27e-3	4	NC	3	NC	1
140			min	-.003	3	-.007	3	-.007	5	-2.533e-5	2	4887.837	2	NC	1
141		14	max	.003	1	.006	2	0	1	2.292e-3	4	NC	3	NC	1
142			min	-.003	3	-.006	3	-.006	5	-2.897e-5	2	5969.558	2	NC	1
143		15	max	.002	1	.004	2	0	1	2.314e-3	4	NC	3	NC	1
144			min	-.002	3	-.004	3	-.005	5	-3.262e-5	2	7598.98	2	NC	1
145		16	max	.002	1	.003	2	0	1	2.336e-3	4	NC	1	NC	1
146			min	-.002	3	-.003	3	-.004	5	-3.627e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	2.358e-3	4	NC	1	NC	1
148			min	-.001	3	-.002	3	-.003	5	-3.991e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.38e-3	4	NC	1	NC	1
150			min	0	3	-.001	3	-.001	5	-4.356e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.402e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.721e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.146e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.104e-3	4	NC	1	NC	1
155		2	max	0	3	.001	2	.006	4	1.916e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-1.093e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.012	4	1.823e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-1.082e-3	4	NC	1	NC	1
159		4	max	0	3	.004	2	.018	4	1.73e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.071e-3	4	NC	1	NC	1
161		5	max	0	3	.005	2	.023	4	1.637e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	1	-1.059e-3	4	8793.896	2	NC	1
163		6	max	0	3	.007	2	.029	4	1.543e-5	1	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	-1.048e-3	4	7046.547	2	NC	1
165		7	max	0	3	.008	2	.035	4	2.182e-5	3	NC	3	NC	1
166			min	-.001	2	-.009	3	0	1	-1.037e-3	4	5851.203	2	NC	1
167		8	max	.001	3	.009	2	.041	4	3.306e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-1.026e-3	4	4974.658	2	NC	1
169		9	max	.001	3	.011	2	.046	4	4.429e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-1.014e-3	4	4300.729	2	NC	1
171		10	max	.001	3	.012	2	.052	4	5.553e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	-.001	1	-1.003e-3	4	3765.011	2	NC	1
173		11	max	.002	3	.014	2	.057	4	6.677e-5	3	NC	3	NC	1
174			min	-.002	2	-.014	3	-.001	1	-9.92e-4	4	3328.818	2	NC	1
175		12	max	.002	3	.016	2	.063	4	7.8e-5	3	NC	3	NC	1
176			min	-.002	2	-.015	3	-.001	1	-9.807e-4	4	2967.38	2	NC	1
177		13	max	.002	3	.017	2	.068	4	8.924e-5	3	NC	3	NC	1
178			min	-.002	2	-.016	3	-.002	1	-9.695e-4	4	2663.972	2	NC	1
179		14	max	.002	3	.019	2	.073	4	1.005e-4	3	NC	3	NC	1
180			min	-.003	2	-.017	3	-.002	1	-9.583e-4	4	2406.794	2	NC	1
181		15	max	.002	3	.021	2	.079	4	1.117e-4	3	NC	3	NC	1
182			min	-.003	2	-.018	3	-.002	1	-9.47e-4	4	2187.204	2	NC	1
183		16	max	.002	3	.023	2	.084	4	1.229e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	-.002	1	-9.358e-4	4	1998.68	2	NC	1
185		17	max	.003	3	.025	2	.089	4	1.342e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	-.002	1	-9.246e-4	4	1836.174	2	NC	1
187		18	max	.003	3	.027	2	.094	4	1.454e-4	3	NC	3	NC	1
188			min	-.003	2	-.02	3	-.002	1	-9.133e-4	4	1695.708	2	NC	1
189		19	max	.003	3	.029	2	.098	4	1.567e-4	3	NC	3	NC	1
190			min	-.004	2	-.021	3	-.002	1	-9.021e-4	4	1574.099	2	NC	1
191	M8	1	max	.007	1	.027	2	.002	1	4.762e-3	4	NC	1	NC	2
192			min	-.001	3	-.019	3	-.104	4	-1.232e-4	3	NC	1	186.466	4
193		2	max	.007	1	.026	2	.002	1	4.762e-3	4	NC	1	NC	2
194			min	-.001	3	-.018	3	-.095	4	-1.232e-4	3	NC	1	203.275	4
195		3	max	.006	1	.024	2	.002	1	4.762e-3	4	NC	1	NC	2
196			min	-.001	3	-.017	3	-.087	4	-1.232e-4	3	NC	1	223.283	4
197		4	max	.006	1	.023	2	.002	1	4.762e-3	4	NC	1	NC	1
198			min	-.001	3	-.016	3	-.078	4	-1.232e-4	3	NC	1	247.331	4
199		5	max	.006	1	.021	2	.002	1	4.762e-3	4	NC	1	NC	1
200			min	0	3	-.015	3	-.07	4	-1.232e-4	3	NC	1	276.568	4
201		6	max	.005	1	.02	2	.001	1	4.762e-3	4	NC	1	NC	1
202			min	0	3	-.014	3	-.062	4	-1.232e-4	3	NC	1	312.589	4
203		7	max	.005	1	.018	2	.001	1	4.762e-3	4	NC	1	NC	1
204			min	0	3	-.013	3	-.054	4	-1.232e-4	3	NC	1	357.667	4
205		8	max	.004	1	.017	2	.001	1	4.762e-3	4	NC	1	NC	1
206			min	0	3	-.012	3	-.047	4	-1.232e-4	3	NC	1	415.132	4
207		9	max	.004	1	.015	2	0	1	4.762e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.039	4	-1.232e-4	3	NC	1	490.016	4
209		10	max	.004	1	.014	2	0	1	4.762e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.033	4	-1.232e-4	3	NC	1	590.235	4
211		11	max	.003	1	.012	2	0	1	4.762e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.027	4	-1.232e-4	3	NC	1	728.822	4
213		12	max	.003	1	.011	2	0	1	4.762e-3	4	NC	1	NC	1





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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	9	0	2	.009	4	1.948e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.019e-3	4	NC	1	5295.59	4
273		4	max	0	9	0	2	.013	4	6.456e-6	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-1.119e-3	4	NC	1	3503.197	4
275		5	max	0	9	0	2	.018	4	-4.691e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	0	1	-1.219e-3	4	NC	1	2610.332	4
277		6	max	0	9	0	2	.022	4	-1.296e-5	12	NC	1	NC	1
278			min	0	2	-.004	3	-.001	1	-1.32e-3	4	NC	1	2076.841	4
279		7	max	0	9	0	2	.027	5	-2.124e-5	12	NC	1	NC	1
280			min	0	2	-.005	3	-.002	1	-1.42e-3	4	NC	1	1717.753	5
281		8	max	0	9	.001	2	.031	5	-2.951e-5	12	NC	1	NC	1
282			min	0	2	-.005	3	-.003	1	-1.52e-3	4	NC	1	1462.244	5
283		9	max	0	9	.002	2	.036	5	-3.778e-5	12	NC	1	NC	1
284			min	0	2	-.006	3	-.004	1	-1.62e-3	4	NC	1	1271.676	5
285		10	max	0	9	.002	2	.041	5	-4.252e-5	10	NC	1	NC	2
286			min	0	2	-.006	3	-.005	1	-1.72e-3	4	NC	1	1124.19	5
287		11	max	0	9	.003	2	.046	5	-4.667e-5	10	NC	1	NC	2
288			min	0	2	-.007	3	-.006	1	-1.821e-3	4	NC	1	1006.682	5
289		12	max	0	9	.003	2	.051	5	-5.081e-5	10	NC	1	NC	2
290			min	0	2	-.007	3	-.007	1	-1.921e-3	4	NC	1	910.821	5
291		13	max	0	9	.004	2	.055	5	-5.495e-5	10	NC	1	NC	2
292			min	0	2	-.007	3	-.008	1	-2.021e-3	4	NC	1	831.058	5
293		14	max	0	9	.005	2	.06	5	-5.91e-5	10	NC	1	NC	2
294			min	0	2	-.007	3	-.009	1	-2.121e-3	4	9827.904	2	763.557	5
295		15	max	0	9	.006	2	.065	5	-6.324e-5	10	NC	3	NC	2
296			min	0	2	-.007	3	-.01	1	-2.221e-3	4	8282.995	2	705.581	5
297		16	max	0	9	.007	2	.07	5	-6.738e-5	10	NC	3	NC	2
298			min	0	2	-.008	3	-.011	1	-2.322e-3	4	7081.77	2	655.128	5
299		17	max	0	9	.008	2	.075	5	-7.153e-5	10	NC	3	NC	2
300			min	0	2	-.008	3	-.012	1	-2.422e-3	4	6138.458	2	610.7	5
301		18	max	.001	9	.009	2	.081	5	-7.567e-5	10	NC	3	NC	3
302			min	0	2	-.008	3	-.013	1	-2.522e-3	4	5391.283	2	571.157	5
303		19	max	.001	9	.01	2	.086	5	-7.982e-5	10	NC	3	NC	3
304			min	0	2	-.008	3	-.014	1	-2.622e-3	4	4795.464	2	535.615	5
305	M12	1	max	.002	1	.009	2	.012	1	6.244e-3	4	NC	1	NC	3
306			min	0	3	-.007	3	-.095	5	7.33e-5	10	NC	1	203.995	5
307		2	max	.002	1	.008	2	.011	1	6.244e-3	4	NC	1	NC	3
308			min	0	3	-.006	3	-.087	5	7.33e-5	10	NC	1	222.38	5
309		3	max	.002	1	.008	2	.01	1	6.244e-3	4	NC	1	NC	3
310			min	0	3	-.006	3	-.079	5	7.33e-5	10	NC	1	244.264	5
311		4	max	.002	1	.007	2	.009	1	6.244e-3	4	NC	1	NC	3
312			min	0	3	-.006	3	-.071	5	7.33e-5	10	NC	1	270.567	5
313		5	max	.002	1	.007	2	.008	1	6.244e-3	4	NC	1	NC	3
314			min	0	3	-.005	3	-.064	5	7.33e-5	10	NC	1	302.545	5
315		6	max	.002	1	.006	2	.007	1	6.244e-3	4	NC	1	NC	3
316			min	0	3	-.005	3	-.057	5	7.33e-5	10	NC	1	341.942	5
317		7	max	.002	1	.006	2	.006	1	6.244e-3	4	NC	1	NC	3
318			min	0	3	-.004	3	-.049	5	7.33e-5	10	NC	1	391.245	5
319		8	max	.002	1	.005	2	.005	1	6.244e-3	4	NC	1	NC	3
320			min	0	3	-.004	3	-.043	5	7.33e-5	10	NC	1	454.094	5
321		9	max	.001	1	.005	2	.004	1	6.244e-3	4	NC	1	NC	2
322			min	0	3	-.004	3	-.036	5	7.33e-5	10	NC	1	535.995	5
323		10	max	.001	1	.004	2	.004	1	6.244e-3	4	NC	1	NC	2
324			min	0	3	-.003	3	-.03	5	7.33e-5	10	NC	1	645.602	5
325		11	max	.001	1	.004	2	.003	1	6.244e-3	4	NC	1	NC	2
326			min	0	3	-.003	3	-.024	5	7.33e-5	10	NC	1	797.169	5
327		12	max	0	1	.003	2	.002	1	6.244e-3	4	NC	1	NC	2



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Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.019	5	7.33e-5	10	NC	1	1015.562	5
329		max	0	1	.003	2	.002	1	6.244e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.014	5	7.33e-5	10	NC	1	1347.418	5
331		max	0	1	.002	2	.001	1	6.244e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.01	5	7.33e-5	10	NC	1	1888.94	5
333		max	0	1	.002	2	0	1	6.244e-3	4	NC	1	NC	1
334		min	0	3	-.001	3	-.007	5	7.33e-5	10	NC	1	2866.142	5
335		max	0	1	.001	2	0	1	6.244e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.004	5	7.33e-5	10	NC	1	4921.53	5
337		max	0	1	0	2	0	1	6.244e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.002	5	7.33e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	6.244e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	7.33e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	6.244e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	7.33e-5	10	NC	1	NC	1
343	M1	max	.006	3	.023	3	.01	5	1.559e-2	1	NC	1	NC	1
344		min	-.007	2	-.028	1	-.005	1	-1.408e-2	3	NC	1	NC	1
345		max	.006	3	.013	3	.014	5	7.409e-3	1	NC	4	NC	2
346		min	-.007	2	-.015	1	-.01	1	-6.967e-3	3	3563.255	1	8394.162	1
347		max	.006	3	.003	3	.018	5	5.263e-4	5	NC	4	NC	2
348		min	-.007	2	-.003	1	-.014	1	-6.211e-4	1	1843.013	1	5089.783	1
349		max	.006	3	.007	1	.023	5	5.287e-4	5	NC	5	NC	2
350		min	-.007	2	-.005	3	-.016	1	-5.188e-4	1	1303.761	1	3518.587	5
351		max	.006	3	.016	1	.028	5	5.311e-4	5	NC	5	NC	2
352		min	-.007	2	-.011	3	-.016	1	-4.165e-4	1	1044.597	1	2521.235	5
353		max	.006	3	.023	1	.034	5	5.335e-4	5	NC	5	NC	2
354		min	-.007	2	-.017	3	-.015	1	-3.142e-4	1	898.151	1	1938.749	5
355		max	.006	3	.029	1	.04	5	5.359e-4	5	NC	5	NC	2
356		min	-.007	2	-.02	3	-.013	1	-2.119e-4	1	809.529	1	1561.31	5
357		max	.006	3	.033	1	.046	5	5.383e-4	5	NC	5	NC	2
358		min	-.007	2	-.023	3	-.011	1	-1.096e-4	1	755.906	1	1299.513	5
359		max	.006	3	.035	1	.052	5	5.406e-4	5	NC	5	NC	1
360		min	-.007	2	-.025	3	-.008	1	-9.63e-6	2	726.629	1	1103.53	4
361		max	.006	3	.036	1	.058	5	5.572e-4	4	NC	5	NC	1
362		min	-.007	2	-.025	3	-.005	1	1.248e-5	10	716.785	1	946.105	4
363		max	.006	3	.035	1	.065	4	5.819e-4	4	NC	5	NC	1
364		min	-.007	2	-.024	3	-.001	1	2.076e-5	10	724.888	1	827.368	4
365		max	.006	3	.032	1	.072	4	6.065e-4	4	NC	5	NC	2
366		min	-.007	2	-.022	3	0	10	2.798e-5	12	752.256	1	735.8	4
367		max	.006	3	.028	1	.079	4	6.311e-4	4	NC	5	NC	2
368		min	-.007	2	-.019	3	0	12	3.021e-5	12	803.573	1	663.99	4
369		max	.006	3	.023	1	.086	4	6.558e-4	4	NC	5	NC	2
370		min	-.007	2	-.015	3	0	12	3.243e-5	12	889.109	1	606.998	4
371		max	.006	3	.015	1	.092	4	6.804e-4	4	NC	5	NC	2
372		min	-.007	2	-.01	3	0	12	3.465e-5	12	1030.887	1	561.432	4
373		max	.006	3	.006	1	.098	4	1.024e-3	4	NC	5	NC	2
374		min	-.007	2	-.004	3	0	12	3.61e-5	12	1281.446	1	524.911	4
375		max	.006	3	.002	3	.103	4	8.947e-3	4	NC	4	NC	2
376		min	-.007	2	-.004	2	0	12	1.366e-5	10	1798.905	1	495.771	4
377		max	.006	3	.01	3	.107	4	8.786e-3	1	NC	4	NC	2
378		min	-.007	2	-.017	1	0	10	-3.222e-3	3	3467.003	1	472.748	4
379		max	.006	3	.018	3	.111	4	1.766e-2	1	NC	1	NC	1
380		min	-.007	2	-.031	1	-.003	1	-6.534e-3	3	NC	1	455.49	4
381	M5	max	.018	3	.068	3	.009	5	5.46e-6	4	NC	1	NC	1
382		min	-.023	2	-.085	1	-.005	1	5.196e-8	10	NC	1	NC	1
383		max	.018	3	.038	3	.013	5	2.578e-4	5	NC	5	NC	1
384		min	-.023	2	-.047	1	-.005	1	-8.423e-5	1	1207.273	1	NC	1



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Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.018	3	.01	3	.018	5	5.06e-4	5	NC	5	NC	1
386			min	-.023	2	-.01	1	-.004	1	-1.672e-4	1	621.4	1	NC	1
387		4	max	.018	3	.02	1	.024	5	5.26e-4	5	NC	5	NC	1
388			min	-.023	2	-.013	3	-.004	1	-1.57e-4	1	438.435	1	NC	1
389		5	max	.018	3	.047	1	.029	5	5.459e-4	5	NC	15	NC	1
390			min	-.023	2	-.032	3	-.003	1	-1.468e-4	1	350.459	1	NC	1
391		6	max	.017	3	.068	1	.035	5	5.659e-4	5	NC	15	NC	1
392			min	-.023	2	-.047	3	-.003	1	-1.365e-4	1	300.665	1	NC	1
393		7	max	.017	3	.085	1	.042	5	5.859e-4	5	NC	15	NC	1
394			min	-.023	2	-.058	3	-.003	1	-1.263e-4	1	270.433	1	NC	1
395		8	max	.017	3	.098	1	.048	5	6.059e-4	5	NC	15	NC	1
396			min	-.023	2	-.066	3	-.002	1	-1.16e-4	1	252.019	1	NC	1
397		9	max	.017	3	.105	1	.055	5	6.258e-4	5	NC	15	NC	1
398			min	-.023	2	-.07	3	-.002	1	-1.058e-4	1	241.803	1	NC	1
399		10	max	.017	3	.107	1	.062	5	6.458e-4	5	NC	15	NC	1
400			min	-.024	2	-.07	3	-.002	1	-9.555e-5	1	238.106	1	NC	1
401		11	max	.017	3	.105	1	.068	5	6.658e-4	5	NC	15	NC	1
402			min	-.024	2	-.068	3	-.002	1	-8.531e-5	1	240.405	1	NC	1
403		12	max	.017	3	.098	1	.075	5	6.857e-4	5	NC	15	NC	1
404			min	-.024	2	-.062	3	-.002	1	-7.506e-5	1	249.116	1	NC	1
405		13	max	.017	3	.085	1	.081	4	7.057e-4	5	NC	15	NC	1
406			min	-.024	2	-.054	3	-.002	1	-6.482e-5	1	265.78	1	NC	1
407		14	max	.017	3	.068	1	.087	4	7.257e-4	5	NC	15	NC	1
408			min	-.024	2	-.042	3	-.002	1	-5.458e-5	1	293.803	1	9572.458	4
409		15	max	.017	3	.046	1	.093	4	7.456e-4	5	NC	15	NC	1
410			min	-.024	2	-.028	3	-.002	1	-4.434e-5	1	340.529	1	9449.865	4
411		16	max	.017	3	.018	1	.099	4	1.08e-3	5	NC	5	NC	1
412			min	-.024	2	-.012	3	-.002	1	-4.135e-5	1	423.609	1	NC	1
413		17	max	.017	3	.007	3	.104	4	8.959e-3	4	NC	5	NC	1
414			min	-.024	2	-.014	1	-.002	1	-2.109e-4	1	597.254	1	NC	1
415		18	max	.017	3	.028	3	.108	4	4.596e-3	4	NC	5	NC	1
416			min	-.024	2	-.053	1	-.003	1	-1.081e-4	1	1157.529	1	NC	1
417		19	max	.017	3	.05	3	.111	4	1.893e-6	5	NC	1	NC	1
418			min	-.024	2	-.094	1	-.003	1	-1.294e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.023	3	.008	5	1.408e-2	3	NC	1	NC	1
420			min	-.007	2	-.028	1	-.006	1	-1.559e-2	1	NC	1	NC	1
421		2	max	.006	3	.013	3	.007	5	6.979e-3	3	NC	4	NC	2
422			min	-.007	2	-.015	1	-.001	1	-7.666e-3	1	3564.13	1	9718.796	1
423		3	max	.006	3	.003	3	.008	4	1.11e-4	1	NC	4	NC	2
424			min	-.007	2	-.003	1	0	3	4.713e-6	12	1843.477	1	6033.638	1
425		4	max	.006	3	.007	1	.01	4	2.5e-5	1	NC	5	NC	2
426			min	-.007	2	-.005	3	0	3	-1.632e-6	3	1304.086	1	5110.954	1
427		5	max	.006	3	.016	1	.012	4	5.603e-6	5	NC	5	NC	2
428			min	-.007	2	-.011	3	-.001	3	-6.099e-5	1	1044.843	1	5062.688	1
429		6	max	.006	3	.023	1	.016	4	-4.051e-6	10	NC	5	NC	2
430			min	-.007	2	-.017	3	-.001	3	-1.47e-4	1	898.347	1	4656.171	4
431		7	max	.006	3	.029	1	.02	4	-1.109e-5	10	NC	5	NC	2
432			min	-.007	2	-.021	3	-.002	3	-2.33e-4	1	809.689	1	3234.183	4
433		8	max	.006	3	.033	1	.026	4	-1.814e-5	10	NC	5	NC	1
434			min	-.007	2	-.023	3	-.002	3	-3.189e-4	1	756.04	1	2389.16	4
435		9	max	.006	3	.035	1	.031	5	-2.518e-5	10	NC	5	NC	1
436			min	-.007	2	-.025	3	-.003	1	-4.049e-4	1	726.742	1	1846.554	4
437		10	max	.006	3	.036	1	.038	5	-3.222e-5	10	NC	5	NC	1
438			min	-.007	2	-.025	3	-.006	1	-4.909e-4	1	716.882	1	1477.343	4
439		11	max	.006	3	.035	1	.046	5	-3.927e-5	10	NC	5	NC	2
440			min	-.007	2	-.024	3	-.009	1	-5.769e-4	1	724.97	1	1214.599	4
441		12	max	.006	3	.032	1	.053	5	-4.626e-5	12	NC	5	NC	2





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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.137	3	.103	1	7.477e-3	1	NC	5	NC	3
500		min	-.111	4	-.351	1	.004	10	-4.159e-3	3	580.047	1	1723.072	1
501	4	max	.002	1	.171	3	.154	1	8.617e-3	1	NC	5	NC	3
502		min	-.111	4	-.443	1	.008	10	-4.746e-3	3	451.592	1	1167.826	1
503	5	max	.002	1	.182	3	.179	1	9.757e-3	1	NC	5	NC	3
504		min	-.111	4	-.47	1	.009	10	-5.334e-3	3	423.005	1	1011.559	1
505	6	max	.002	1	.171	3	.169	1	1.09e-2	1	NC	5	NC	10
506		min	-.111	4	-.436	1	.007	10	-5.921e-3	3	458.915	1	1064.706	1
507	7	max	.002	1	.142	3	.129	1	1.204e-2	1	NC	5	NC	3
508		min	-.111	4	-.352	1	.002	10	-6.508e-3	3	579.294	1	1388.143	1
509	8	max	.003	1	.103	3	.068	1	1.318e-2	1	NC	5	NC	3
510		min	-.111	4	-.241	1	-.004	10	-7.096e-3	3	882.544	1	2536.123	1
511	9	max	.003	1	.067	3	.019	3	1.432e-2	1	NC	5	NC	1
512		min	-.111	4	-.14	1	-.013	2	-7.683e-3	3	1701.296	1	NC	1
513	10	max	.003	1	.05	3	.017	3	1.546e-2	1	NC	4	NC	1
514		min	-.111	4	-.094	1	-.024	2	-8.271e-3	3	2948.025	1	NC	1
515	11	max	.003	1	.067	3	.017	3	1.432e-2	1	NC	5	NC	1
516		min	-.111	4	-.14	1	-.012	2	-7.683e-3	3	1701.296	1	NC	1
517	12	max	.003	1	.103	3	.066	1	1.318e-2	1	NC	5	NC	3
518		min	-.111	4	-.241	1	-.004	10	-7.095e-3	3	882.544	1	2607.979	1
519	13	max	.003	1	.142	3	.126	1	1.204e-2	1	NC	5	NC	3
520		min	-.111	4	-.352	1	.002	10	-6.507e-3	3	579.294	1	1418.389	1
521	14	max	.003	1	.171	3	.166	1	1.09e-2	1	NC	5	NC	3
522		min	-.111	4	-.436	1	.003	15	-5.919e-3	3	458.915	1	1086.237	1
523	15	max	.003	1	.182	3	.175	1	9.758e-3	1	NC	5	NC	3
524		min	-.111	4	-.47	1	-.002	5	-5.332e-3	3	423.006	1	1032.622	1
525	16	max	.003	1	.171	3	.15	1	8.618e-3	1	NC	5	NC	3
526		min	-.111	4	-.443	1	-.009	5	-4.744e-3	3	451.592	1	1195.1	1
527	17	max	.003	1	.137	3	.1	1	7.479e-3	1	NC	5	NC	3
528		min	-.111	4	-.351	1	-.013	5	-4.156e-3	3	580.048	1	1772.811	1
529	18	max	.003	1	.083	3	.039	1	6.339e-3	1	NC	5	NC	2
530		min	-.111	4	-.207	1	-.011	5	-3.568e-3	3	1055.472	1	4185.19	1
531	19	max	.003	1	.018	3	.006	3	5.2e-3	1	NC	1	NC	1
532		min	-.111	4	-.031	1	-.007	2	-2.98e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.17e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.241e-4	5	NC	1	NC	1
535	2	max	0	3	-.002	15	.011	4	8.028e-4	3	NC	5	NC	1
536		min	0	5	-.017	1	0	3	-7.403e-4	1	6009.63	6	8696.984	4
537	3	max	0	3	-.003	15	.024	4	1.289e-3	3	NC	5	NC	1
538		min	-.002	5	-.033	1	-.003	3	-1.41e-3	1	3058.096	6	4055.457	4
539	4	max	0	3	-.004	15	.037	4	1.774e-3	3	NC	5	NC	3
540		min	-.003	5	-.048	1	-.006	3	-2.08e-3	1	2098.032	6	2628.758	4
541	5	max	0	3	-.005	15	.05	4	2.26e-3	3	NC	15	NC	9
542		min	-.004	5	-.061	1	-.01	3	-2.75e-3	1	1637.116	6	1982.544	4
543	6	max	0	3	-.006	15	.06	4	2.746e-3	3	NC	15	9139.436	10
544		min	-.005	5	-.073	1	-.014	3	-3.419e-3	1	1377.806	6	1642.479	4
545	7	max	0	3	-.007	15	.067	4	3.232e-3	3	NC	15	7199.328	10
546		min	-.006	5	-.082	1	-.019	3	-4.089e-3	1	1221.865	6	1457.069	4
547	8	max	0	3	-.008	15	.072	4	3.718e-3	3	NC	15	5970.869	10
548		min	-.007	5	-.089	1	-.023	3	-4.759e-3	1	1128.278	6	1366.583	4
549	9	max	0	3	-.008	15	.073	4	4.203e-3	3	NC	15	5163.299	10
550		min	-.008	5	-.093	1	-.027	3	-5.429e-3	1	1077.903	6	1346.936	4
551	10	max	0	3	-.008	15	.07	4	4.689e-3	3	NC	15	4629.393	10
552		min	-.009	5	-.095	1	-.031	3	-6.098e-3	1	1061.967	6	1392.913	4
553	11	max	0	3	-.007	15	.065	4	5.175e-3	3	NC	15	4291.59	10
554		min	-.01	5	-.094	1	-.033	3	-6.768e-3	1	1077.903	6	1514.796	4
555	12	max	0	3	-.007	15	.056	4	5.661e-3	3	NC	15	4111.71	10





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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.