



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	100 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 15.70 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.669 k-ft
$M_z$ =	0.262 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>83%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.321 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.415 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.253 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>12%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.670 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>29%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.423 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<u>26%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

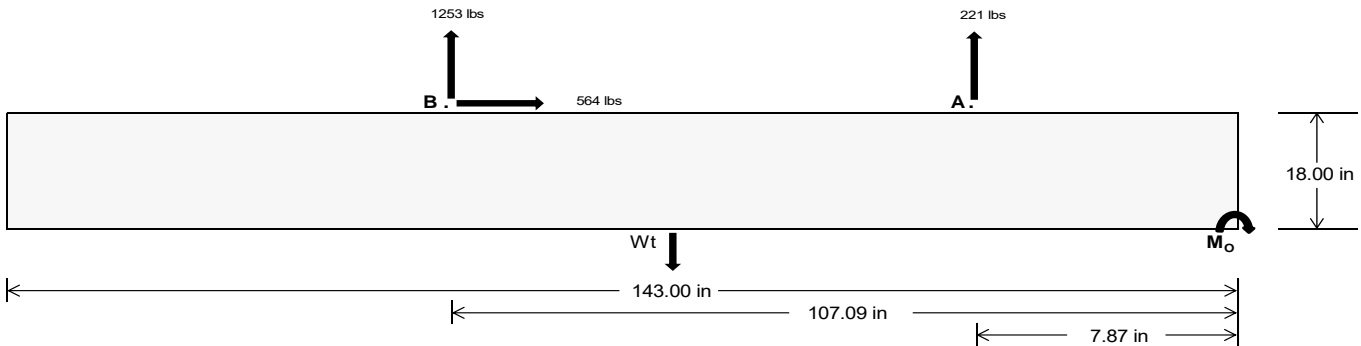
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>930.42</u>	<u>5224.53</u> k
Compressive Load =		<u>4229.04</u>	<u>4774.48</u> k
Lateral Load =		<u>14.08</u>	<u>2346.09</u> k
Moment (Weak Axis) =		<u>0.03</u>	<u>0.01</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 146073.1$  in-lbs  
Resisting Force Required = 2042.98 lbs  
S.F. = 1.67  
Weight Required = 3404.97 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 563.89 lbs  
Friction = 0.4  
Weight Required = 1409.73 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 563.89 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1493 lbs	1493 lbs	1493 lbs	1493 lbs	1450 lbs	1450 lbs	1450 lbs	1450 lbs	2085 lbs	2085 lbs	2085 lbs	2085 lbs	-441 lbs	-441 lbs	-441 lbs	-441 lbs
$F_B$	1604 lbs	1604 lbs	1604 lbs	1604 lbs	1772 lbs	1772 lbs	1772 lbs	1772 lbs	2401 lbs	2401 lbs	2401 lbs	2401 lbs	-2506 lbs	-2506 lbs	-2506 lbs	-2506 lbs
$F_V$	156 lbs	156 lbs	156 lbs	156 lbs	1007 lbs	1007 lbs	1007 lbs	1007 lbs	861 lbs	861 lbs	861 lbs	861 lbs	-1128 lbs	-1128 lbs	-1128 lbs	-1128 lbs
$P_{total}$	10657 lbs	10873 lbs	11089 lbs	11305 lbs	10781 lbs	10997 lbs	11213 lbs	11429 lbs	12046 lbs	12262 lbs	12478 lbs	12694 lbs	1588 lbs	1718 lbs	1847 lbs	1977 lbs
$M$	3396 lbs-ft	3396 lbs-ft	3396 lbs-ft	3396 lbs-ft	3944 lbs-ft	3944 lbs-ft	3944 lbs-ft	3944 lbs-ft	5225 lbs-ft	5225 lbs-ft	5225 lbs-ft	5225 lbs-ft	3399 lbs-ft	3399 lbs-ft	3399 lbs-ft	3399 lbs-ft
$e$	0.32 ft	0.31 ft	0.31 ft	0.30 ft	0.37 ft	0.36 ft	0.35 ft	0.35 ft	0.43 ft	0.43 ft	0.42 ft	0.41 ft	2.14 ft	1.98 ft	1.84 ft	1.72 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	257.4 psf	256.3 psf	255.3 psf	254.3 psf	253.1 psf	252.1 psf	251.1 psf	250.3 psf	270.9 psf	269.4 psf	268.0 psf	266.7 psf	0.0 psf	0.2 psf	3.7 psf	7.0 psf
$f_{max}$	355.8 psf	352.0 psf	348.3 psf	344.9 psf	367.3 psf	363.2 psf	359.2 psf	355.5 psf	422.3 psf	416.6 psf	411.2 psf	406.1 psf	95.1 psf	95.9 psf	96.9 psf	97.7 psf

Maximum Bearing Pressure = 422 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

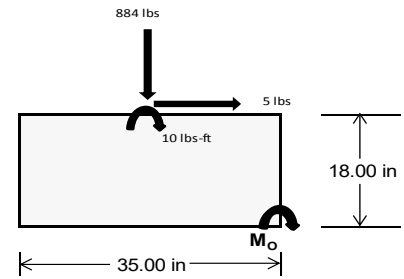
#### Overturning Check

$M_o = 1271.6 \text{ ft-lbs}$   
 Resisting Force Required = 871.99 lbs  
 S.F. = 1.67  
 Weight Required = 1453.31 lbs  
 Minimum Width = **35 in**  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	247 lbs	628 lbs	247 lbs	884 lbs	2540 lbs	884 lbs	72 lbs	184 lbs	72 lbs
$F_v$	1 lbs	0 lbs	1 lbs	5 lbs	0 lbs	5 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9605 lbs	7560 lbs	9605 lbs	9793 lbs	7560 lbs	9793 lbs	2809 lbs	7560 lbs	2809 lbs
$M$	5 lbs-ft	0 lbs-ft	5 lbs-ft	18 lbs-ft	0 lbs-ft	18 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	276.1 psf	217.5 psf	276.1 psf	280.7 psf	217.5 psf	280.7 psf	80.8 psf	217.5 psf	80.8 psf
$f_{max}$	276.6 psf	217.5 psf	276.6 psf	282.8 psf	217.5 psf	282.8 psf	80.8 psf	217.5 psf	80.8 psf



Maximum Bearing Pressure = 283 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements: 143in long x 28in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.**

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.664 k
Allowable Uplift =	1.214 k
Utilization =	<u>55%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.999 k
Allowable Uplift =	4.357 k
Utilization =	<u>46%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.253 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

#### Rear Strut

Maximum Axial Load =	3.594 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>48%</u>

#### Diagonal Strut

Maximum Axial Load =	1.799 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>24%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 102 \text{ in}$$

$$J = 0.432$$

$$282.18$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 27.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102$$

$$J = 0.432$$

$$179.449$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.0$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$\begin{aligned} Rb/t &= 18.1 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= \phi b [Bt - Dt \sqrt{(Rb/t)}] \\ \phi F_L &= 31.1 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 7.4 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 35.2 \\ m &= 0.68 \\ C_0 &= 41.067 \\ Cc &= 43.717 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 73.8 \\ \phi F_L &= 1.3\phi y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 29.0 \text{ ksi} \\ I_x &= 984962 \text{ mm}^4 \\ &= 2.366 \text{ in}^4 \\ y &= 43.717 \text{ mm} \\ S_x &= 1.375 \text{ in}^3 \\ M_{max} St &= 3.323 \text{ k-ft} \end{aligned}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 16.2 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 40 \\ Cc &= 40 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3\phi y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 33.3 \text{ ksi} \\ I_y &= 923544 \text{ mm}^4 \\ &= 2.219 \text{ in}^4 \\ x &= 40 \text{ mm} \\ S_y &= 1.409 \text{ in}^3 \\ M_{max} Wk &= 3.904 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.9

$$\begin{aligned} b/t &= 16.2 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \\ b/t &= 7.4 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 18.1 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi c [Bt - Dt \sqrt{(Rb/t)}] \\ \phi F_L &= 31.09 \text{ ksi} \\ \phi F_L &= 31.09 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{max} &= 58.55 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.41345$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77788$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 13.6277 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-54.088	-54.088	0	0
2	M14	y	-54.088	-54.088	0	0
3	M15	y	-84.995	-84.995	0	0
4	M16	y	-84.995	-84.995	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	123.63	123.63	0	0
2	M14	y	94.783	94.783	0	0
3	M15	y	51.512	51.512	0	0
4	M16	y	51.512	51.512	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 20° 100mph 30psf 8.5ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	67.52	1	242.618	1	.14	3	.014	2	-.004	15	.756	3
28			min	2.435	15	-293.02	3	-27.501	1	0	15	-.121	1	-.545	1
29		15	max	67.52	1	99.406	1	9.75	1	.014	2	-.004	12	.947	3
30			min	2.435	15	-111.759	3	.309	10	0	15	-.13	1	-.707	1
31		16	max	67.52	1	69.502	3	47.001	1	.014	2	-.003	12	.967	3
32			min	2.435	15	-43.806	1	1.695	15	0	15	-.103	1	-.733	1
33		17	max	67.52	1	250.763	3	84.251	1	.014	2	.001	3	.816	3
34			min	2.435	15	-187.018	1	3.021	15	0	15	-.041	1	-.624	1
35		18	max	67.52	1	432.024	3	121.502	1	.014	2	.056	1	.494	3
36			min	2.435	15	-330.23	1	4.348	15	0	15	.002	15	-.38	1
37		19	max	67.52	1	613.285	3	158.753	1	.014	2	.188	1	0	1
38			min	2.435	15	-473.442	1	5.675	15	0	15	.007	15	0	3
39	M14	1	max	40.603	1	533.011	1	-5.905	15	.011	3	.225	1	0	1
40			min	1.466	15	-495.036	3	-165.196	1	-.015	1	.008	15	0	3
41		2	max	40.603	1	389.799	1	-4.578	15	.011	3	.086	1	.403	3
42			min	1.466	15	-357.367	3	-127.945	1	-.015	1	.003	15	-.436	1
43		3	max	40.603	1	246.587	1	-3.252	15	.011	3	.003	3	.675	3
44			min	1.466	15	-219.697	3	-90.695	1	-.015	1	-.017	1	-.736	1
45		4	max	40.603	1	103.375	1	-1.925	15	.011	3	-.002	12	.818	3
46			min	1.466	15	-82.027	3	-53.444	1	-.015	1	-.085	1	-.902	1
47		5	max	40.603	1	55.643	3	-.598	15	.011	3	-.004	12	.83	3
48			min	1.466	15	-39.837	1	-16.193	1	-.015	1	-.118	1	-.932	1
49		6	max	40.603	1	193.313	3	21.058	1	.011	3	-.004	15	.712	3
50			min	1.466	15	-183.049	1	-.51	3	-.015	1	-.116	1	-.826	1
51		7	max	40.603	1	330.983	3	58.308	1	.011	3	-.003	15	.465	3
52			min	1.466	15	-326.261	1	1.075	12	-.015	1	-.078	1	-.586	1
53		8	max	40.603	1	468.653	3	95.559	1	.011	3	.001	10	.087	3
54			min	1.466	15	-469.473	1	2.424	12	-.015	1	-.005	1	-.21	1
55		9	max	40.603	1	606.323	3	132.81	1	.011	3	.102	1	.301	1
56			min	1.466	15	-612.685	1	3.772	12	-.015	1	0	3	-.42	3
57		10	max	40.603	1	743.993	3	170.061	1	.011	3	.245	1	.947	1
58			min	1.466	15	-755.897	1	5.121	12	-.015	1	.004	12	-1.058	3
59		11	max	40.603	1	612.685	1	-3.772	12	.015	1	.102	1	.301	1
60			min	1.466	15	-606.323	3	-132.81	1	-.011	3	0	3	-.42	3
61		12	max	40.603	1	469.473	1	-2.424	12	.015	1	.001	10	.087	3
62			min	1.466	15	-468.653	3	-95.559	1	-.011	3	-.005	1	-.21	1
63		13	max	40.603	1	326.261	1	-1.075	12	.015	1	-.003	15	.465	3
64			min	1.466	15	-330.983	3	-58.308	1	-.011	3	-.078	1	-.586	1
65		14	max	40.603	1	183.049	1	.51	3	.015	1	-.004	15	.712	3
66			min	1.466	15	-193.313	3	-21.058	1	-.011	3	-.116	1	-.826	1
67		15	max	40.603	1	39.837	1	16.193	1	.015	1	-.004	12	.83	3
68			min	1.466	15	-55.643	3	.598	15	-.011	3	-.118	1	-.932	1
69		16	max	40.603	1	82.027	3	53.444	1	.015	1	-.002	12	.818	3
70			min	1.466	15	-103.375	1	1.925	15	-.011	3	-.085	1	-.902	1
71		17	max	40.603	1	219.697	3	90.695	1	.015	1	.003	3	.675	3
72			min	1.466	15	-246.587	1	3.252	15	-.011	3	-.017	1	-.736	1
73		18	max	40.603	1	357.367	3	127.945	1	.015	1	.086	1	.403	3
74			min	1.466	15	-389.799	1	4.578	15	-.011	3	.003	15	-.436	1
75		19	max	40.603	1	495.036	3	165.196	1	.015	1	.225	1	0	1
76			min	1.466	15	-533.011	1	5.905	15	-.011	3	.008	15	0	3
77	M15	1	max	-1.558	15	615.677	2	-5.903	15	.016	1	.225	1	0	2
78			min	-43.044	1	-275.008	3	-165.165	1	-.009	3	.008	15	0	3
79		2	max	-1.558	15	446.949	2	-4.576	15	.016	1	.086	1	.226	3
80			min	-43.044	1	-202.726	3	-127.914	1	-.009	3	.003	15	-.502	2
81		3	max	-1.558	15	278.669	1	-3.249	15	.016	1	.003	3	.383	3
82			min	-43.044	1	-130.443	3	-90.664	1	-.009	3	-.017	1	-.844	2
83		4	max	-1.558	15	112.105	1	-1.922	15	.016	1	-.002	12	.472	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-43.044	1	-58.16	3	-53.413	1	-.009	3	-.085	1	-1.027	2
85		5	max	-1.558	15	14.122	3	-.595	15	.016	1	-.004	12	.493	3
86			min	-43.044	1	-59.235	2	-16.162	1	-.009	3	-.118	1	-1.053	1
87		6	max	-1.558	15	86.405	3	21.088	1	.016	1	-.004	15	.445	3
88			min	-43.044	1	-227.963	2	-.371	3	-.009	3	-.116	1	-.923	1
89		7	max	-1.558	15	158.688	3	58.339	1	.016	1	-.003	15	.33	3
90			min	-43.044	1	-396.691	2	1.162	12	-.009	3	-.078	1	-.635	1
91		8	max	-1.558	15	230.97	3	95.59	1	.016	1	0	10	.146	3
92			min	-43.044	1	-565.419	2	2.511	12	-.009	3	-.005	1	-.191	1
93		9	max	-1.558	15	303.253	3	132.841	1	.016	1	.102	1	.448	2
94			min	-43.044	1	-734.147	2	3.859	12	-.009	3	0	3	-.107	3
95		10	max	-1.558	15	375.535	3	170.091	1	.016	1	.245	1	1.221	2
96			min	-43.044	1	-902.875	2	5.208	12	-.006	12	.005	12	-.427	3
97		11	max	-1.558	15	734.147	2	-3.859	12	.009	3	.102	1	.448	2
98			min	-43.044	1	-303.253	3	-132.841	1	-.016	1	0	3	-.107	3
99		12	max	-1.558	15	565.419	2	-2.511	12	.009	3	0	10	.146	3
100			min	-43.044	1	-230.97	3	-95.59	1	-.016	1	-.005	1	-.191	1
101		13	max	-1.558	15	396.691	2	-1.162	12	.009	3	-.003	15	.33	3
102			min	-43.044	1	-158.688	3	-58.339	1	-.016	1	-.078	1	-.635	1
103		14	max	-1.558	15	227.963	2	.371	3	.009	3	-.004	15	.445	3
104			min	-43.044	1	-86.405	3	-21.088	1	-.016	1	-.116	1	-.923	1
105		15	max	-1.558	15	59.235	2	16.162	1	.009	3	-.004	12	.493	3
106			min	-43.044	1	-14.122	3	.595	15	-.016	1	-.118	1	-1.053	1
107		16	max	-1.558	15	58.16	3	53.413	1	.009	3	-.002	12	.472	3
108			min	-43.044	1	-112.105	1	1.922	15	-.016	1	-.085	1	-1.027	2
109		17	max	-1.558	15	130.443	3	90.664	1	.009	3	.003	3	.383	3
110			min	-43.044	1	-278.669	1	3.249	15	-.016	1	-.017	1	-.844	2
111		18	max	-1.558	15	202.726	3	127.914	1	.009	3	.086	1	.226	3
112			min	-43.044	1	-446.949	2	4.576	15	-.016	1	.003	15	-.502	2
113		19	max	-1.558	15	275.008	3	165.165	1	.009	3	.225	1	0	2
114			min	-43.044	1	-615.677	2	5.903	15	-.016	1	.008	15	0	3
115	M16	1	max	-2.71	15	562.08	2	-5.688	15	.012	1	.191	1	0	2
116			min	-75.074	1	-236.051	3	-159.237	1	-.011	3	.007	15	0	3
117		2	max	-2.71	15	393.352	2	-4.361	15	.012	1	.058	1	.189	3
118			min	-75.074	1	-163.768	3	-121.986	1	-.011	3	.002	15	-.451	2
119		3	max	-2.71	15	224.624	2	-3.034	15	.012	1	0	3	.309	3
120			min	-75.074	1	-91.485	3	-84.735	1	-.011	3	-.04	1	-.743	2
121		4	max	-2.71	15	55.896	2	-1.708	15	.012	1	-.003	12	.362	3
122			min	-75.074	1	-19.203	3	-47.485	1	-.011	3	-.102	1	-.875	2
123		5	max	-2.71	15	53.08	3	-.381	15	.012	1	-.004	12	.346	3
124			min	-75.074	1	-113.097	1	-10.234	1	-.011	3	-.129	1	-.849	2
125		6	max	-2.71	15	125.363	3	27.017	1	.012	1	-.004	15	.261	3
126			min	-75.074	1	-281.56	2	.244	12	-.011	3	-.121	1	-.662	2
127		7	max	-2.71	15	197.645	3	64.268	1	.012	1	-.003	15	.109	3
128			min	-75.074	1	-450.288	2	1.593	12	-.011	3	-.078	1	-.317	2
129		8	max	-2.71	15	269.928	3	101.518	1	.012	1	.002	2	.197	1
130			min	-75.074	1	-619.016	2	2.941	12	-.011	3	-.003	3	-.112	3
131		9	max	-2.71	15	342.21	3	138.769	1	.012	1	.114	1	.855	1
132			min	-75.074	1	-787.744	2	4.29	12	-.011	3	.002	12	-.401	3
133		10	max	-2.71	15	414.493	3	176.02	1	.012	1	.262	1	1.676	2
134			min	-75.074	1	-956.472	2	5.638	12	-.011	3	.006	12	-.758	3
135		11	max	-2.71	15	787.744	2	-4.29	12	.011	3	.114	1	.855	1
136			min	-75.074	1	-342.21	3	-138.769	1	-.012	1	.002	12	-.401	3
137		12	max	-2.71	15	619.016	2	-2.941	12	.011	3	.002	2	.197	1
138			min	-75.074	1	-269.928	3	-101.518	1	-.012	1	-.003	3	-.112	3
139		13	max	-2.71	15	450.288	2	-1.593	12	.011	3	-.003	15	.109	3
140			min	-75.074	1	-197.645	3	-64.268	1	-.012	1	-.078	1	-.317	2



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-2.71	15	281.56	2	-.244	12	.011	3	-.004	15	.261	3
142			min	-75.074	1	-125.363	3	-27.017	1	-.012	1	-.121	1	-.662	2
143		15	max	-2.71	15	113.097	1	10.234	1	.011	3	-.004	12	.346	3
144			min	-75.074	1	-53.08	3	.381	15	-.012	1	-.129	1	-.849	2
145		16	max	-2.71	15	19.203	3	47.485	1	.011	3	-.003	12	.362	3
146			min	-75.074	1	-55.896	2	1.708	15	-.012	1	-.102	1	-.875	2
147		17	max	-2.71	15	91.485	3	84.735	1	.011	3	0	3	.309	3
148			min	-75.074	1	-224.624	2	3.034	15	-.012	1	-.04	1	-.743	2
149		18	max	-2.71	15	163.768	3	121.986	1	.011	3	.058	1	.189	3
150			min	-75.074	1	-393.352	2	4.361	15	-.012	1	.002	15	-.451	2
151		19	max	-2.71	15	236.051	3	159.237	1	.011	3	.191	1	0	2
152			min	-75.074	1	-562.08	2	5.688	15	-.012	1	.007	15	0	3
153	M2	1	max	1100.812	1	2.157	4	.796	1	0	3	0	3	0	1
154			min	-1129.603	3	.507	15	.029	15	0	1	0	1	0	1
155		2	max	1101.228	1	2.148	4	.796	1	0	3	0	1	0	15
156			min	-1129.291	3	.505	15	.029	15	0	1	0	15	0	4
157		3	max	1101.644	1	2.139	4	.796	1	0	3	0	1	0	15
158			min	-1128.979	3	.503	15	.029	15	0	1	0	15	-.001	4
159		4	max	1102.06	1	2.13	4	.796	1	0	3	0	1	0	15
160			min	-1128.667	3	.501	15	.029	15	0	1	0	15	-.002	4
161		5	max	1102.475	1	2.122	4	.796	1	0	3	0	1	0	15
162			min	-1128.355	3	.499	15	.029	15	0	1	0	15	-.002	4
163		6	max	1102.891	1	2.113	4	.796	1	0	3	.001	1	0	15
164			min	-1128.043	3	.497	15	.029	15	0	1	0	15	-.003	4
165		7	max	1103.307	1	2.104	4	.796	1	0	3	.001	1	0	15
166			min	-1127.731	3	.495	15	.029	15	0	1	0	15	-.004	4
167		8	max	1103.723	1	2.096	4	.796	1	0	3	.002	1	0	15
168			min	-1127.419	3	.493	15	.029	15	0	1	0	15	-.004	4
169		9	max	1104.139	1	2.087	4	.796	1	0	3	.002	1	-.001	15
170			min	-1127.107	3	.491	15	.029	15	0	1	0	15	-.005	4
171		10	max	1104.555	1	2.078	4	.796	1	0	3	.002	1	-.001	15
172			min	-1126.796	3	.489	15	.029	15	0	1	0	15	-.005	4
173		11	max	1104.971	1	2.069	4	.796	1	0	3	.002	1	-.001	15
174			min	-1126.484	3	.487	15	.029	15	0	1	0	15	-.006	4
175		12	max	1105.387	1	2.061	4	.796	1	0	3	.002	1	-.002	15
176			min	-1126.172	3	.485	15	.029	15	0	1	0	15	-.007	4
177		13	max	1105.802	1	2.052	4	.796	1	0	3	.003	1	-.002	15
178			min	-1125.86	3	.482	15	.029	15	0	1	0	15	-.007	4
179		14	max	1106.218	1	2.043	4	.796	1	0	3	.003	1	-.002	15
180			min	-1125.548	3	.48	15	.029	15	0	1	0	15	-.008	4
181		15	max	1106.634	1	2.035	4	.796	1	0	3	.003	1	-.002	15
182			min	-1125.236	3	.478	15	.029	15	0	1	0	15	-.008	4
183		16	max	1107.05	1	2.026	4	.796	1	0	3	.003	1	-.002	15
184			min	-1124.924	3	.476	15	.029	15	0	1	0	15	-.009	4
185		17	max	1107.466	1	2.017	4	.796	1	0	3	.004	1	-.002	15
186			min	-1124.612	3	.474	15	.029	15	0	1	0	15	-.009	4
187		18	max	1107.882	1	2.008	4	.796	1	0	3	.004	1	-.002	15
188			min	-1124.3	3	.472	15	.029	15	0	1	0	15	-.01	4
189		19	max	1108.298	1	2	4	.796	1	0	3	.004	1	-.002	15
190			min	-1123.988	3	.47	15	.029	15	0	1	0	15	-.01	4
191	M3	1	max	458.376	2	9.101	4	.188	1	0	3	0	1	.01	4
192			min	-591.214	3	2.139	15	.007	15	0	1	0	15	.002	15
193		2	max	458.206	2	8.226	4	.188	1	0	3	0	1	.006	4
194			min	-591.341	3	1.934	15	.007	15	0	1	0	15	.002	15
195		3	max	458.036	2	7.352	4	.188	1	0	3	0	1	.003	2
196			min	-591.469	3	1.728	15	.007	15	0	1	0	15	0	3
197		4	max	457.865	2	6.477	4	.188	1	0	3	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-591.597	3	1.523	15	.007	15	0	1	0	15	-.002	3
199		5	max	457.695	2	5.603	4	.188	1	0	3	0	1	0	15
200			min	-591.725	3	1.317	15	.007	15	0	1	0	15	-.003	4
201		6	max	457.525	2	4.728	4	.188	1	0	3	0	1	-.001	15
202			min	-591.852	3	1.112	15	.007	15	0	1	0	15	-.006	4
203		7	max	457.354	2	3.854	4	.188	1	0	3	0	1	-.002	15
204			min	-591.98	3	.906	15	.007	15	0	1	0	15	-.008	4
205		8	max	457.184	2	2.98	4	.188	1	0	3	0	1	-.002	15
206			min	-592.108	3	.7	15	.007	15	0	1	0	15	-.01	4
207		9	max	457.014	2	2.105	4	.188	1	0	3	0	1	-.003	15
208			min	-592.236	3	.495	15	.007	15	0	1	0	15	-.011	4
209		10	max	456.843	2	1.231	4	.188	1	0	3	0	1	-.003	15
210			min	-592.363	3	.289	15	.007	15	0	1	0	15	-.012	4
211		11	max	456.673	2	.418	2	.188	1	0	3	0	1	-.003	15
212			min	-592.491	3	.014	3	.007	15	0	1	0	15	-.012	4
213		12	max	456.503	2	-.122	15	.188	1	0	3	.001	1	-.003	15
214			min	-592.619	3	-.518	4	.007	15	0	1	0	15	-.012	4
215		13	max	456.332	2	-.327	15	.188	1	0	3	.001	1	-.003	15
216			min	-592.747	3	-1.393	4	.007	15	0	1	0	15	-.011	4
217		14	max	456.162	2	-.533	15	.188	1	0	3	.001	1	-.002	15
218			min	-592.874	3	-2.267	4	.007	15	0	1	0	15	-.011	4
219		15	max	455.992	2	-.738	15	.188	1	0	3	.001	1	-.002	15
220			min	-593.002	3	-3.142	4	.007	15	0	1	0	15	-.009	4
221		16	max	455.821	2	-.944	15	.188	1	0	3	.001	1	-.002	15
222			min	-593.13	3	-4.016	4	.007	15	0	1	0	15	-.008	4
223		17	max	455.651	2	-1.15	15	.188	1	0	3	.001	1	-.001	15
224			min	-593.258	3	-4.89	4	.007	15	0	1	0	15	-.005	4
225		18	max	455.481	2	-1.355	15	.188	1	0	3	.002	1	0	15
226			min	-593.385	3	-5.765	4	.007	15	0	1	0	15	-.003	4
227		19	max	455.31	2	-1.561	15	.188	1	0	3	.002	1	0	1
228			min	-593.513	3	-6.639	4	.007	15	0	1	0	15	0	1
229	M4	1	max	1168.75	1	0	1	-.402	15	0	1	0	1	0	1
230			min	-207.525	3	0	1	-11.217	1	0	1	0	15	0	1
231		2	max	1168.921	1	0	1	-.402	15	0	1	0	12	0	1
232			min	-207.398	3	0	1	-11.217	1	0	1	0	1	0	1
233		3	max	1169.091	1	0	1	-.402	15	0	1	0	15	0	1
234			min	-207.27	3	0	1	-11.217	1	0	1	-.002	1	0	1
235		4	max	1169.261	1	0	1	-.402	15	0	1	0	15	0	1
236			min	-207.142	3	0	1	-11.217	1	0	1	-.003	1	0	1
237		5	max	1169.432	1	0	1	-.402	15	0	1	0	15	0	1
238			min	-207.014	3	0	1	-11.217	1	0	1	-.004	1	0	1
239		6	max	1169.602	1	0	1	-.402	15	0	1	0	15	0	1
240			min	-206.887	3	0	1	-11.217	1	0	1	-.005	1	0	1
241		7	max	1169.772	1	0	1	-.402	15	0	1	0	15	0	1
242			min	-206.759	3	0	1	-11.217	1	0	1	-.007	1	0	1
243		8	max	1169.943	1	0	1	-.402	15	0	1	0	15	0	1
244			min	-206.631	3	0	1	-11.217	1	0	1	-.008	1	0	1
245		9	max	1170.113	1	0	1	-.402	15	0	1	0	15	0	1
246			min	-206.503	3	0	1	-11.217	1	0	1	-.009	1	0	1
247		10	max	1170.283	1	0	1	-.402	15	0	1	0	15	0	1
248			min	-206.376	3	0	1	-11.217	1	0	1	-.011	1	0	1
249		11	max	1170.454	1	0	1	-.402	15	0	1	0	15	0	1
250			min	-206.248	3	0	1	-11.217	1	0	1	-.012	1	0	1
251		12	max	1170.624	1	0	1	-.402	15	0	1	0	15	0	1
252			min	-206.12	3	0	1	-11.217	1	0	1	-.013	1	0	1
253		13	max	1170.794	1	0	1	-.402	15	0	1	0	15	0	1
254			min	-205.992	3	0	1	-11.217	1	0	1	-.014	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1170.965	1	0	1	-.402	15	0	1	0	15	0	1
256		min	-205.865	3	0	1	-11.217	1	0	1	-.016	1	0	1
257	15	max	1171.135	1	0	1	-.402	15	0	1	0	15	0	1
258		min	-205.737	3	0	1	-11.217	1	0	1	-.017	1	0	1
259	16	max	1171.305	1	0	1	-.402	15	0	1	0	15	0	1
260		min	-205.609	3	0	1	-11.217	1	0	1	-.018	1	0	1
261	17	max	1171.476	1	0	1	-.402	15	0	1	0	15	0	1
262		min	-205.481	3	0	1	-11.217	1	0	1	-.02	1	0	1
263	18	max	1171.646	1	0	1	-.402	15	0	1	0	15	0	1
264		min	-205.354	3	0	1	-11.217	1	0	1	-.021	1	0	1
265	19	max	1171.816	1	0	1	-.402	15	0	1	0	15	0	1
266		min	-205.226	3	0	1	-11.217	1	0	1	-.022	1	0	1
267	M6	1	max	3415.321	1	2.511	2	0	1	0	0	1	0	1
268		min	-3593.803	3	.209	12	0	1	0	1	0	1	0	1
269	2	max	3415.737	1	2.504	2	0	1	0	1	0	1	0	12
270		min	-3593.491	3	.205	12	0	1	0	1	0	1	0	2
271	3	max	3416.152	1	2.497	2	0	1	0	1	0	1	0	12
272		min	-3593.179	3	.202	12	0	1	0	1	0	1	-.001	2
273	4	max	3416.568	1	2.49	2	0	1	0	1	0	1	0	12
274		min	-3592.867	3	.199	12	0	1	0	1	0	1	-.002	2
275	5	max	3416.984	1	2.484	2	0	1	0	1	0	1	0	12
276		min	-3592.555	3	.195	12	0	1	0	1	0	1	-.003	2
277	6	max	3417.4	1	2.477	2	0	1	0	1	0	1	0	12
278		min	-3592.243	3	.192	12	0	1	0	1	0	1	-.003	2
279	7	max	3417.816	1	2.47	2	0	1	0	1	0	1	0	12
280		min	-3591.931	3	.188	12	0	1	0	1	0	1	-.004	2
281	8	max	3418.232	1	2.463	2	0	1	0	1	0	1	0	12
282		min	-3591.62	3	.185	12	0	1	0	1	0	1	-.005	2
283	9	max	3418.648	1	2.456	2	0	1	0	1	0	1	0	12
284		min	-3591.308	3	.182	12	0	1	0	1	0	1	-.006	2
285	10	max	3419.064	1	2.45	2	0	1	0	1	0	1	0	12
286		min	-3590.996	3	.178	12	0	1	0	1	0	1	-.006	2
287	11	max	3419.48	1	2.443	2	0	1	0	1	0	1	0	12
288		min	-3590.684	3	.175	12	0	1	0	1	0	1	-.007	2
289	12	max	3419.895	1	2.436	2	0	1	0	1	0	1	0	12
290		min	-3590.372	3	.171	12	0	1	0	1	0	1	-.008	2
291	13	max	3420.311	1	2.429	2	0	1	0	1	0	1	0	12
292		min	-3590.06	3	.168	12	0	1	0	1	0	1	-.008	2
293	14	max	3420.727	1	2.422	2	0	1	0	1	0	1	0	12
294		min	-3589.748	3	.165	12	0	1	0	1	0	1	-.009	2
295	15	max	3421.143	1	2.416	2	0	1	0	1	0	1	0	12
296		min	-3589.436	3	.161	12	0	1	0	1	0	1	-.01	2
297	16	max	3421.559	1	2.409	2	0	1	0	1	0	1	0	12
298		min	-3589.124	3	.158	12	0	1	0	1	0	1	-.01	2
299	17	max	3421.975	1	2.402	2	0	1	0	1	0	1	0	12
300		min	-3588.812	3	.154	12	0	1	0	1	0	1	-.011	2
301	18	max	3422.391	1	2.395	2	0	1	0	1	0	1	0	12
302		min	-3588.5	3	.151	12	0	1	0	1	0	1	-.012	2
303	19	max	3422.807	1	2.388	2	0	1	0	1	0	1	0	12
304		min	-3588.189	3	.148	12	0	1	0	1	0	1	-.012	2
305	M7	1	max	1670.416	2	9.139	4	0	1	0	0	1	.012	2
306		min	-1796.892	3	2.144	15	0	1	0	1	0	1	0	12
307	2	max	1670.246	2	8.264	4	0	1	0	1	0	1	.009	2
308		min	-1797.02	3	1.939	15	0	1	0	1	0	1	-.001	3
309	3	max	1670.076	2	7.39	4	0	1	0	1	0	1	.006	2
310		min	-1797.148	3	1.733	15	0	1	0	1	0	1	-.003	3
311	4	max	1669.905	2	6.515	4	0	1	0	1	0	1	.003	2



Company : Schletter, Inc.  
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Nov 4, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-1797.276	3	1.528	15	0	1	0	1	0	1	-.005	3
313	5	max	1669.735	2	5.641	4	0	1	0	1	0	1	0	2
314		min	-1797.403	3	1.322	15	0	1	0	1	0	1	-.006	3
315	6	max	1669.565	2	4.766	4	0	1	0	1	0	1	-.001	15
316		min	-1797.531	3	1.117	15	0	1	0	1	0	1	-.007	3
317	7	max	1669.394	2	3.892	4	0	1	0	1	0	1	-.002	15
318		min	-1797.659	3	.911	15	0	1	0	1	0	1	-.008	3
319	8	max	1669.224	2	3.017	4	0	1	0	1	0	1	-.002	15
320		min	-1797.787	3	.706	15	0	1	0	1	0	1	-.009	4
321	9	max	1669.054	2	2.143	4	0	1	0	1	0	1	-.002	15
322		min	-1797.914	3	.446	12	0	1	0	1	0	1	-.011	4
323	10	max	1668.883	2	1.451	2	0	1	0	1	0	1	-.003	15
324		min	-1798.042	3	.106	12	0	1	0	1	0	1	-.011	4
325	11	max	1668.713	2	.769	2	0	1	0	1	0	1	-.003	15
326		min	-1798.17	3	-.39	3	0	1	0	1	0	1	-.012	4
327	12	max	1668.543	2	.088	2	0	1	0	1	0	1	-.003	15
328		min	-1798.298	3	-.901	3	0	1	0	1	0	1	-.012	4
329	13	max	1668.372	2	-.322	15	0	1	0	1	0	1	-.003	15
330		min	-1798.425	3	-1.412	3	0	1	0	1	0	1	-.011	4
331	14	max	1668.202	2	-.528	15	0	1	0	1	0	1	-.002	15
332		min	-1798.553	3	-2.229	4	0	1	0	1	0	1	-.01	4
333	15	max	1668.032	2	-.733	15	0	1	0	1	0	1	-.002	15
334		min	-1798.681	3	-3.104	4	0	1	0	1	0	1	-.009	4
335	16	max	1667.861	2	-.939	15	0	1	0	1	0	1	-.002	15
336		min	-1798.809	3	-3.978	4	0	1	0	1	0	1	-.008	4
337	17	max	1667.691	2	-1.144	15	0	1	0	1	0	1	-.001	15
338		min	-1798.936	3	-4.853	4	0	1	0	1	0	1	-.005	4
339	18	max	1667.52	2	-1.35	15	0	1	0	1	0	1	0	15
340		min	-1799.064	3	-5.727	4	0	1	0	1	0	1	-.003	4
341	19	max	1667.35	2	-1.555	15	0	1	0	1	0	1	0	1
342		min	-1799.192	3	-6.601	4	0	1	0	1	0	1	0	1
343	M8	1	max	3250.043	1	0	1	0	1	0	1	0	1	1
344		min	-718.006	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3250.213	1	0	1	0	1	0	1	0	1	0	1
346		min	-717.878	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3250.383	1	0	1	0	1	0	1	0	1	0	1
348		min	-717.75	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3250.554	1	0	1	0	1	0	1	0	1	0	1
350		min	-717.623	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3250.724	1	0	1	0	1	0	1	0	1	0	1
352		min	-717.495	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3250.894	1	0	1	0	1	0	1	0	1	0	1
354		min	-717.367	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3251.065	1	0	1	0	1	0	1	0	1	0	1
356		min	-717.239	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3251.235	1	0	1	0	1	0	1	0	1	0	1
358		min	-717.112	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3251.405	1	0	1	0	1	0	1	0	1	0	1
360		min	-716.984	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3251.576	1	0	1	0	1	0	1	0	1	0	1
362		min	-716.856	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3251.746	1	0	1	0	1	0	1	0	1	0	1
364		min	-716.728	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3251.916	1	0	1	0	1	0	1	0	1	0	1
366		min	-716.601	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3252.087	1	0	1	0	1	0	1	0	1	0	1
368		min	-716.473	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3252.257	1	0	1	0	1	0	1	0	1	0	1
370			min	-716.345	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3252.428	1	0	1	0	1	0	1	0	1	0	1
372			min	-716.217	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3252.598	1	0	1	0	1	0	1	0	1	0	1
374			min	-716.09	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3252.768	1	0	1	0	1	0	1	0	1	0	1
376			min	-715.962	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3252.939	1	0	1	0	1	0	1	0	1	0	1
378			min	-715.834	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3253.109	1	0	1	0	1	0	1	0	1	0	1
380			min	-715.706	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1100.812	1	2.157	4	-.029	15	0	1	0	1	0	1
382			min	-1129.603	3	.507	15	-.796	1	0	3	0	3	0	1
383		2	max	1101.228	1	2.148	4	-.029	15	0	1	0	15	0	15
384			min	-1129.291	3	.505	15	-.796	1	0	3	0	1	0	4
385		3	max	1101.644	1	2.139	4	-.029	15	0	1	0	15	0	15
386			min	-1128.979	3	.503	15	-.796	1	0	3	0	1	-.001	4
387		4	max	1102.06	1	2.13	4	-.029	15	0	1	0	15	0	15
388			min	-1128.667	3	.501	15	-.796	1	0	3	0	1	-.002	4
389		5	max	1102.475	1	2.122	4	-.029	15	0	1	0	15	0	15
390			min	-1128.355	3	.499	15	-.796	1	0	3	0	1	-.002	4
391		6	max	1102.891	1	2.113	4	-.029	15	0	1	0	15	0	15
392			min	-1128.043	3	.497	15	-.796	1	0	3	-.001	1	-.003	4
393		7	max	1103.307	1	2.104	4	-.029	15	0	1	0	15	0	15
394			min	-1127.731	3	.495	15	-.796	1	0	3	-.001	1	-.004	4
395		8	max	1103.723	1	2.096	4	-.029	15	0	1	0	15	0	15
396			min	-1127.419	3	.493	15	-.796	1	0	3	-.002	1	-.004	4
397		9	max	1104.139	1	2.087	4	-.029	15	0	1	0	15	-.001	15
398			min	-1127.107	3	.491	15	-.796	1	0	3	-.002	1	-.005	4
399		10	max	1104.555	1	2.078	4	-.029	15	0	1	0	15	-.001	15
400			min	-1126.796	3	.489	15	-.796	1	0	3	-.002	1	-.005	4
401		11	max	1104.971	1	2.069	4	-.029	15	0	1	0	15	-.001	15
402			min	-1126.484	3	.487	15	-.796	1	0	3	-.002	1	-.006	4
403		12	max	1105.387	1	2.061	4	-.029	15	0	1	0	15	-.002	15
404			min	-1126.172	3	.485	15	-.796	1	0	3	-.002	1	-.007	4
405		13	max	1105.802	1	2.052	4	-.029	15	0	1	0	15	-.002	15
406			min	-1125.86	3	.482	15	-.796	1	0	3	-.003	1	-.007	4
407		14	max	1106.218	1	2.043	4	-.029	15	0	1	0	15	-.002	15
408			min	-1125.548	3	.48	15	-.796	1	0	3	-.003	1	-.008	4
409		15	max	1106.634	1	2.035	4	-.029	15	0	1	0	15	-.002	15
410			min	-1125.236	3	.478	15	-.796	1	0	3	-.003	1	-.008	4
411		16	max	1107.05	1	2.026	4	-.029	15	0	1	0	15	-.002	15
412			min	-1124.924	3	.476	15	-.796	1	0	3	-.003	1	-.009	4
413		17	max	1107.466	1	2.017	4	-.029	15	0	1	0	15	-.002	15
414			min	-1124.612	3	.474	15	-.796	1	0	3	-.004	1	-.009	4
415		18	max	1107.882	1	2.008	4	-.029	15	0	1	0	15	-.002	15
416			min	-1124.3	3	.472	15	-.796	1	0	3	-.004	1	-.01	4
417		19	max	1108.298	1	2	4	-.029	15	0	1	0	15	-.002	15
418			min	-1123.988	3	.47	15	-.796	1	0	3	-.004	1	-.01	4
419	M11	1	max	458.376	2	9.101	4	-.007	15	0	1	0	15	.01	4
420			min	-591.214	3	2.139	15	-.188	1	0	3	0	1	.002	15
421		2	max	458.206	2	8.226	4	-.007	15	0	1	0	15	.006	4
422			min	-591.341	3	1.934	15	-.188	1	0	3	0	1	.002	15
423		3	max	458.036	2	7.352	4	-.007	15	0	1	0	15	.003	2
424			min	-591.469	3	1.728	15	-.188	1	0	3	0	1	0	3
425		4	max	457.865	2	6.477	4	-.007	15	0	1	0	15	0	2



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-591.597	3	1.523	15	-.188	1	0	3	0	1	-.002	3
427		5	max	457.695	2	5.603	4	-.007	15	0	1	0	15	0	15
428			min	-591.725	3	1.317	15	-.188	1	0	3	0	1	-.003	4
429		6	max	457.525	2	4.728	4	-.007	15	0	1	0	15	-.001	15
430			min	-591.852	3	1.112	15	-.188	1	0	3	0	1	-.006	4
431		7	max	457.354	2	3.854	4	-.007	15	0	1	0	15	-.002	15
432			min	-591.98	3	.906	15	-.188	1	0	3	0	1	-.008	4
433		8	max	457.184	2	2.98	4	-.007	15	0	1	0	15	-.002	15
434			min	-592.108	3	.7	15	-.188	1	0	3	0	1	-.01	4
435		9	max	457.014	2	2.105	4	-.007	15	0	1	0	15	-.003	15
436			min	-592.236	3	.495	15	-.188	1	0	3	0	1	-.011	4
437		10	max	456.843	2	1.231	4	-.007	15	0	1	0	15	-.003	15
438			min	-592.363	3	.289	15	-.188	1	0	3	0	1	-.012	4
439		11	max	456.673	2	.418	2	-.007	15	0	1	0	15	-.003	15
440			min	-592.491	3	.014	3	-.188	1	0	3	0	1	-.012	4
441		12	max	456.503	2	-.122	15	-.007	15	0	1	0	15	-.003	15
442			min	-592.619	3	-.518	4	-.188	1	0	3	-.001	1	-.012	4
443		13	max	456.332	2	-.327	15	-.007	15	0	1	0	15	-.003	15
444			min	-592.747	3	-1.393	4	-.188	1	0	3	-.001	1	-.011	4
445		14	max	456.162	2	-.533	15	-.007	15	0	1	0	15	-.002	15
446			min	-592.874	3	-2.267	4	-.188	1	0	3	-.001	1	-.011	4
447		15	max	455.992	2	-.738	15	-.007	15	0	1	0	15	-.002	15
448			min	-593.002	3	-3.142	4	-.188	1	0	3	-.001	1	-.009	4
449		16	max	455.821	2	-.944	15	-.007	15	0	1	0	15	-.002	15
450			min	-593.13	3	-4.016	4	-.188	1	0	3	-.001	1	-.008	4
451		17	max	455.651	2	-1.15	15	-.007	15	0	1	0	15	-.001	15
452			min	-593.258	3	-4.89	4	-.188	1	0	3	-.001	1	-.005	4
453		18	max	455.481	2	-1.355	15	-.007	15	0	1	0	15	0	15
454			min	-593.385	3	-5.765	4	-.188	1	0	3	-.002	1	-.003	4
455		19	max	455.31	2	-1.561	15	-.007	15	0	1	0	15	0	1
456			min	-593.513	3	-6.639	4	-.188	1	0	3	-.002	1	0	1
457	M12	1	max	1168.75	1	0	1	11.217	1	0	1	0	15	0	1
458			min	-207.525	3	0	1	.402	15	0	1	0	1	0	1
459		2	max	1168.921	1	0	1	11.217	1	0	1	0	1	0	1
460			min	-207.398	3	0	1	.402	15	0	1	0	12	0	1
461		3	max	1169.091	1	0	1	11.217	1	0	1	.002	1	0	1
462			min	-207.27	3	0	1	.402	15	0	1	0	15	0	1
463		4	max	1169.261	1	0	1	11.217	1	0	1	.003	1	0	1
464			min	-207.142	3	0	1	.402	15	0	1	0	15	0	1
465		5	max	1169.432	1	0	1	11.217	1	0	1	.004	1	0	1
466			min	-207.014	3	0	1	.402	15	0	1	0	15	0	1
467		6	max	1169.602	1	0	1	11.217	1	0	1	.005	1	0	1
468			min	-206.887	3	0	1	.402	15	0	1	0	15	0	1
469		7	max	1169.772	1	0	1	11.217	1	0	1	.007	1	0	1
470			min	-206.759	3	0	1	.402	15	0	1	0	15	0	1
471		8	max	1169.943	1	0	1	11.217	1	0	1	.008	1	0	1
472			min	-206.631	3	0	1	.402	15	0	1	0	15	0	1
473		9	max	1170.113	1	0	1	11.217	1	0	1	.009	1	0	1
474			min	-206.503	3	0	1	.402	15	0	1	0	15	0	1
475		10	max	1170.283	1	0	1	11.217	1	0	1	.011	1	0	1
476			min	-206.376	3	0	1	.402	15	0	1	0	15	0	1
477		11	max	1170.454	1	0	1	11.217	1	0	1	.012	1	0	1
478			min	-206.248	3	0	1	.402	15	0	1	0	15	0	1
479		12	max	1170.624	1	0	1	11.217	1	0	1	.013	1	0	1
480			min	-206.12	3	0	1	.402	15	0	1	0	15	0	1
481		13	max	1170.794	1	0	1	11.217	1	0	1	.014	1	0	1
482			min	-205.992	3	0	1	.402	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1170.965	1	0	1	11.217	1	0	1	.016	1	0	1
484		min	-205.865	3	0	1	.402	15	0	1	0	15	0	1
485	15	max	1171.135	1	0	1	11.217	1	0	1	.017	1	0	1
486		min	-205.737	3	0	1	.402	15	0	1	0	15	0	1
487	16	max	1171.305	1	0	1	11.217	1	0	1	.018	1	0	1
488		min	-205.609	3	0	1	.402	15	0	1	0	15	0	1
489	17	max	1171.476	1	0	1	11.217	1	0	1	.02	1	0	1
490		min	-205.481	3	0	1	.402	15	0	1	0	15	0	1
491	18	max	1171.646	1	0	1	11.217	1	0	1	.021	1	0	1
492		min	-205.354	3	0	1	.402	15	0	1	0	15	0	1
493	19	max	1171.816	1	0	1	11.217	1	0	1	.022	1	0	1
494		min	-205.226	3	0	1	.402	15	0	1	0	15	0	1
495	M1	1	max	158.758	1	613.246	3	-2.435	15	0	.188	1	0	15
496		min	5.675	15	-471.334	1	-67.419	1	0	3	.007	15	-.014	2
497	2	max	159.334	1	612.058	3	-2.435	15	0	1	.147	1	.279	1
498		min	5.849	15	-472.917	1	-67.419	1	0	3	.005	15	-.383	3
499	3	max	380.783	3	561.378	1	-2.407	15	0	3	.105	1	.562	1
500		min	-250.684	2	-459.519	3	-66.839	1	0	1	.004	15	-.751	3
501	4	max	381.215	3	559.795	1	-2.407	15	0	3	.063	1	.214	1
502		min	-250.108	2	-460.707	3	-66.839	1	0	1	.002	15	-.465	3
503	5	max	381.647	3	558.212	1	-2.407	15	0	3	.022	1	-.005	15
504		min	-249.532	2	-461.894	3	-66.839	1	0	1	0	15	-.179	3
505	6	max	382.079	3	556.629	1	-2.407	15	0	3	0	15	.108	3
506		min	-248.955	2	-463.081	3	-66.839	1	0	1	-.02	1	-.479	1
507	7	max	382.511	3	555.046	1	-2.407	15	0	3	-.002	15	.396	3
508		min	-248.379	2	-464.269	3	-66.839	1	0	1	-.061	1	-.824	1
509	8	max	382.944	3	553.462	1	-2.407	15	0	3	-.004	15	.685	3
510		min	-247.803	2	-465.456	3	-66.839	1	0	1	-.103	1	-1.168	1
511	9	max	393.999	3	39.948	2	-3.873	15	0	9	.066	1	.8	3
512		min	-183.139	2	.482	15	-107.487	1	0	3	.002	15	-1.329	1
513	10	max	394.432	3	38.365	2	-3.873	15	0	9	0	15	.781	3
514		min	-182.563	2	.004	15	-107.487	1	0	3	-.001	1	-1.343	1
515	11	max	394.864	3	36.782	2	-3.873	15	0	9	-.002	15	.762	3
516		min	-181.987	2	-1.93	4	-107.487	1	0	3	-.068	1	-1.355	1
517	12	max	405.781	3	306.241	3	-2.315	15	0	1	.101	1	.666	3
518		min	-117.279	2	-596.854	1	-64.472	1	0	3	.004	15	-1.198	1
519	13	max	406.213	3	305.054	3	-2.315	15	0	1	.061	1	.477	3
520		min	-116.702	2	-598.437	1	-64.472	1	0	3	.002	15	-.827	1
521	14	max	406.646	3	303.867	3	-2.315	15	0	1	.021	1	.288	3
522		min	-116.126	2	-600.02	1	-64.472	1	0	3	0	15	-.455	1
523	15	max	407.078	3	302.679	3	-2.315	15	0	1	0	15	.099	3
524		min	-115.55	2	-601.604	1	-64.472	1	0	3	-.019	1	-.082	1
525	16	max	407.51	3	301.492	3	-2.315	15	0	1	-.002	15	.315	2
526		min	-114.974	2	-603.187	1	-64.472	1	0	3	-.059	1	-.088	3
527	17	max	407.942	3	300.305	3	-2.315	15	0	1	-.004	15	.679	2
528		min	-114.398	2	-604.77	1	-64.472	1	0	3	-.099	1	-.275	3
529	18	max	-5.862	15	564.365	2	-2.71	15	0	3	-.005	15	.34	2
530		min	-159.809	1	-234.939	3	-75.17	1	0	2	-.144	1	-.135	3
531	19	max	-5.688	15	562.781	2	-2.71	15	0	3	-.007	15	.011	3
532		min	-159.233	1	-236.126	3	-75.17	1	0	2	-.191	1	-.012	1
533	M5	1	max	352.997	1	2036.073	3	0	1	0	0	1	.028	2
534		min	10.734	12	-1621.451	1	0	1	0	1	0	1	0	15
535	2	max	353.574	1	2034.885	3	0	1	0	1	0	1	1.035	1
536		min	11.022	12	-1623.035	1	0	1	0	1	0	1	-1.258	3
537	3	max	1189.099	3	1568.54	1	0	1	0	1	0	1	2.008	1
538		min	-831.078	2	-1386.765	3	0	1	0	1	0	1	-2.483	3
539	4	max	1189.531	3	1566.956	1	0	1	0	1	0	1	1.035	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-830.502	2	-1387.952	3	0	1	0	1	0	1	-1.622	3
541		5	max	1189.963	3	1565.373	1	0	1	0	1	0	1	.063	1
542			min	-829.926	2	-1389.14	3	0	1	0	1	0	1	-.76	3
543		6	max	1190.395	3	1563.79	1	0	1	0	1	0	1	.102	3
544			min	-829.349	2	-1390.327	3	0	1	0	1	0	1	-.908	1
545		7	max	1190.827	3	1562.207	1	0	1	0	1	0	1	.965	3
546			min	-828.773	2	-1391.514	3	0	1	0	1	0	1	-1.878	1
547		8	max	1191.259	3	1560.624	1	0	1	0	1	0	1	1.829	3
548			min	-828.197	2	-1392.702	3	0	1	0	1	0	1	-2.847	1
549		9	max	1206.856	3	134.093	2	0	1	0	1	0	1	2.111	3
550			min	-691.83	2	.479	15	0	1	0	1	0	1	-3.232	1
551		10	max	1207.288	3	132.509	2	0	1	0	1	0	1	2.039	3
552			min	-691.254	2	.001	15	0	1	0	1	0	1	-3.277	1
553		11	max	1207.72	3	130.926	2	0	1	0	1	0	1	1.968	3
554			min	-690.678	2	-1.766	4	0	1	0	1	0	1	-3.321	1
555		12	max	1223.593	3	890.177	3	0	1	0	1	0	1	1.722	3
556			min	-554.399	2	-1703.435	1	0	1	0	1	0	1	-2.954	1
557		13	max	1224.025	3	888.989	3	0	1	0	1	0	1	1.17	3
558			min	-553.823	2	-1705.018	1	0	1	0	1	0	1	-1.896	1
559		14	max	1224.457	3	887.802	3	0	1	0	1	0	1	.618	3
560			min	-553.247	2	-1706.602	1	0	1	0	1	0	1	-.837	1
561		15	max	1224.889	3	886.615	3	0	1	0	1	0	1	.293	2
562			min	-552.671	2	-1708.185	1	0	1	0	1	0	1	0	15
563		16	max	1225.321	3	885.427	3	0	1	0	1	0	1	1.326	2
564			min	-552.094	2	-1709.768	1	0	1	0	1	0	1	-.482	3
565		17	max	1225.754	3	884.24	3	0	1	0	1	0	1	2.36	2
566			min	-551.518	2	-1711.351	1	0	1	0	1	0	1	-1.031	3
567		18	max	-11.565	12	1917.506	2	0	1	0	1	0	1	1.208	2
568			min	-352.624	1	-828.112	3	0	1	0	1	0	1	-.536	3
569		19	max	-11.276	12	1915.923	2	0	1	0	1	0	1	.023	1
570			min	-352.048	1	-829.3	3	0	1	0	1	0	1	-.022	3
571	M9	1	max	158.758	1	613.246	3	67.419	1	0	3	-.007	15	0	15
572			min	5.675	15	-471.334	1	2.435	15	0	1	-.188	1	-.014	2
573		2	max	159.334	1	612.058	3	67.419	1	0	3	-.005	15	.279	1
574			min	5.849	15	-472.917	1	2.435	15	0	1	-.147	1	-.383	3
575		3	max	380.783	3	561.378	1	66.839	1	0	1	-.004	15	.562	1
576			min	-250.684	2	-459.519	3	2.407	15	0	3	-.105	1	-.751	3
577		4	max	381.215	3	559.795	1	66.839	1	0	1	-.002	15	.214	1
578			min	-250.108	2	-460.707	3	2.407	15	0	3	-.063	1	-.465	3
579		5	max	381.647	3	558.212	1	66.839	1	0	1	0	15	-.005	15
580			min	-249.532	2	-461.894	3	2.407	15	0	3	-.022	1	-.179	3
581		6	max	382.079	3	556.629	1	66.839	1	0	1	.02	1	.108	3
582			min	-248.955	2	-463.081	3	2.407	15	0	3	0	15	-.479	1
583		7	max	382.511	3	555.046	1	66.839	1	0	1	.061	1	.396	3
584			min	-248.379	2	-464.269	3	2.407	15	0	3	.002	15	-.824	1
585		8	max	382.944	3	553.462	1	66.839	1	0	1	.103	1	.685	3
586			min	-247.803	2	-465.456	3	2.407	15	0	3	.004	15	-1.168	1
587		9	max	393.999	3	39.948	2	107.487	1	0	3	-.002	15	.8	3
588			min	-183.139	2	.482	15	3.873	15	0	9	-.066	1	-1.329	1
589		10	max	394.432	3	38.365	2	107.487	1	0	3	.001	1	.781	3
590			min	-182.563	2	.004	15	3.873	15	0	9	0	15	-1.343	1
591		11	max	394.864	3	36.782	2	107.487	1	0	3	.068	1	.762	3
592			min	-181.987	2	-1.93	4	3.873	15	0	9	.002	15	-1.355	1
593		12	max	405.781	3	306.241	3	64.472	1	0	3	-.004	15	.666	3
594			min	-117.279	2	-596.854	1	2.315	15	0	1	-.101	1	-1.198	1
595		13	max	406.213	3	305.054	3	64.472	1	0	3	-.002	15	.477	3
596			min	-116.702	2	-598.437	1	2.315	15	0	1	-.061	1	-.827	1



Company : Schletter, Inc.  
Designer : HCV  
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Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	406.646	3	303.867	3	64.472	1	0	3	0	15	.288	3
598		min	-116.126	2	-600.02	1	2.315	15	0	1	-.021	1	-.455	1
599	15	max	407.078	3	302.679	3	64.472	1	0	3	.019	1	.099	3
600		min	-115.55	2	-601.604	1	2.315	15	0	1	0	15	-.082	1
601	16	max	407.51	3	301.492	3	64.472	1	0	3	.059	1	.315	2
602		min	-114.974	2	-603.187	1	2.315	15	0	1	.002	15	-.088	3
603	17	max	407.942	3	300.305	3	64.472	1	0	3	.099	1	.679	2
604		min	-114.398	2	-604.77	1	2.315	15	0	1	.004	15	-.275	3
605	18	max	-5.862	15	564.365	2	75.17	1	0	2	.144	1	.34	2
606		min	-159.809	1	-234.939	3	2.71	15	0	3	.005	15	-.135	3
607	19	max	-5.688	15	562.781	2	75.17	1	0	2	.191	1	.011	3
608		min	-159.233	1	-236.126	3	2.71	15	0	3	.007	15	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.199	1	.008	3	1.332e-2	1	NC	1	NC	1
2			min	0	15	-.042	3	-.004	2	-2.664e-3	3	NC	1	NC	1
3		2	max	0	1	.139	3	.025	1	1.449e-2	1	NC	5	NC	2
4			min	0	15	.003	15	0	10	-2.451e-3	3	1124.87	3	8327.414	1
5		3	max	0	1	.286	3	.058	1	1.567e-2	1	NC	5	NC	3
6			min	0	15	-.01	9	.002	10	-2.238e-3	3	620.477	3	3554.487	1
7		4	max	0	1	.377	3	.085	1	1.684e-2	1	NC	5	NC	3
8			min	0	15	-.046	1	.003	15	-2.024e-3	3	486.347	3	2403.796	1
9		5	max	0	1	.4	3	.099	1	1.801e-2	1	NC	5	NC	3
10			min	0	15	-.039	1	.004	15	-1.811e-3	3	460.839	3	2079.451	1
11		6	max	0	1	.358	3	.094	1	1.919e-2	1	NC	5	NC	3
12			min	0	15	-.004	9	.002	10	-1.598e-3	3	510.198	3	2186.684	1
13		7	max	0	1	.262	3	.072	1	2.036e-2	1	NC	4	NC	3
14			min	0	15	.003	15	0	10	-1.385e-3	3	669.941	3	2848.156	1
15		8	max	0	1	.231	2	.04	1	2.153e-2	1	NC	4	NC	2
16			min	0	15	.006	15	-.005	10	-1.172e-3	3	1119.05	3	5191.012	1
17		9	max	0	1	.32	1	.023	3	2.271e-2	1	NC	4	NC	1
18			min	0	15	.009	15	-.009	2	-9.592e-4	3	1623.507	2	NC	1
19		10	max	0	1	.363	1	.023	3	2.388e-2	1	NC	5	NC	1
20			min	0	1	-.022	3	-.016	2	-7.461e-4	3	1240.186	1	NC	1
21		11	max	0	15	.32	1	.023	3	2.271e-2	1	NC	4	NC	1
22			min	0	1	.009	15	-.009	2	-9.592e-4	3	1623.507	2	NC	1
23		12	max	0	15	.231	2	.04	1	2.153e-2	1	NC	4	NC	2
24			min	0	1	.006	15	-.005	10	-1.172e-3	3	1119.05	3	5191.012	1
25		13	max	0	15	.262	3	.072	1	2.036e-2	1	NC	4	NC	3
26			min	0	1	.003	15	0	10	-1.385e-3	3	669.941	3	2848.156	1
27		14	max	0	15	.358	3	.094	1	1.919e-2	1	NC	5	NC	3
28			min	0	1	-.004	9	.002	10	-1.598e-3	3	510.198	3	2186.684	1
29		15	max	0	15	.4	3	.099	1	1.801e-2	1	NC	5	NC	3
30			min	0	1	-.039	1	.004	15	-1.811e-3	3	460.839	3	2079.451	1
31		16	max	0	15	.377	3	.085	1	1.684e-2	1	NC	5	NC	3
32			min	0	1	-.046	1	.003	15	-2.024e-3	3	486.347	3	2403.796	1
33		17	max	0	15	.286	3	.058	1	1.567e-2	1	NC	5	NC	3
34			min	0	1	-.01	9	.002	10	-2.238e-3	3	620.477	3	3554.487	1
35		18	max	0	15	.139	3	.025	1	1.449e-2	1	NC	5	NC	2
36			min	0	1	.003	15	0	10	-2.451e-3	3	1124.87	3	8327.414	1
37		19	max	0	15	.199	1	.008	3	1.332e-2	1	NC	1	NC	1
38			min	0	1	-.042	3	-.004	2	-2.664e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.329	3	.007	3	7.886e-3	1	NC	1	NC	1
40			min	0	15	-.605	1	-.004	2	-5.039e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.546	3	.016	1	9.144e-3	1	NC	5	NC	1
42			min	0	15	-.879	1	-.001	10	-5.945e-3	3	744.328	1	NC	1
43		3	max	0	1	.735	3	.044	1	1.04e-2	1	NC	5	NC	2
44			min	0	15	-1.123	1	0	10	-6.852e-3	3	393.748	1	4674.178	1
45		4	max	0	1	.877	3	.071	1	1.166e-2	1	NC	15	NC	3
46			min	0	15	-1.316	1	.003	15	-7.758e-3	3	286.998	1	2921.151	1
47		5	max	0	1	.961	3	.085	1	1.292e-2	1	9871.161	15	NC	3
48			min	0	15	-1.446	1	.003	15	-8.664e-3	3	242.713	1	2418.078	1
49		6	max	0	1	.988	3	.083	1	1.418e-2	1	9204.39	15	NC	3
50			min	0	15	-1.51	1	.002	10	-9.571e-3	3	225.402	1	2471.102	1
51		7	max	0	1	.965	3	.065	1	1.544e-2	1	9182.585	15	NC	2
52			min	0	15	-1.517	1	0	10	-1.048e-2	3	223.706	1	3151.485	1
53		8	max	0	1	.91	3	.037	1	1.67e-2	1	9594.443	15	NC	2
54			min	0	15	-1.484	1	-.004	10	-1.138e-2	3	232.317	1	5631.273	1
55		9	max	0	1	.85	3	.021	3	1.795e-2	1	NC	15	NC	1
56			min	0	15	-1.436	1	-.008	2	-1.229e-2	3	245.713	1	NC	1
57		10	max	0	1	.82	3	.02	3	1.921e-2	1	NC	15	NC	1
58			min	0	1	-1.41	1	-.014	2	-1.32e-2	3	253.583	1	NC	1
59		11	max	0	15	.85	3	.021	3	1.795e-2	1	NC	15	NC	1
60			min	0	1	-1.436	1	-.008	2	-1.229e-2	3	245.713	1	NC	1
61		12	max	0	15	.91	3	.037	1	1.67e-2	1	9594.443	15	NC	2
62			min	0	1	-1.484	1	-.004	10	-1.138e-2	3	232.317	1	5631.273	1
63		13	max	0	15	.965	3	.065	1	1.544e-2	1	9182.585	15	NC	2
64			min	0	1	-1.517	1	0	10	-1.048e-2	3	223.706	1	3151.485	1
65		14	max	0	15	.988	3	.083	1	1.418e-2	1	9204.39	15	NC	3
66			min	0	1	-1.51	1	.002	10	-9.571e-3	3	225.402	1	2471.102	1
67		15	max	0	15	.961	3	.085	1	1.292e-2	1	9871.161	15	NC	3
68			min	0	1	-1.446	1	.003	15	-8.664e-3	3	242.713	1	2418.078	1
69		16	max	0	15	.877	3	.071	1	1.166e-2	1	NC	15	NC	3
70			min	0	1	-1.316	1	.003	15	-7.758e-3	3	286.998	1	2921.151	1
71		17	max	0	15	.735	3	.044	1	1.04e-2	1	NC	5	NC	2
72			min	0	1	-1.123	1	0	10	-6.852e-3	3	393.748	1	4674.178	1
73		18	max	0	15	.546	3	.016	1	9.144e-3	1	NC	5	NC	1
74			min	0	1	-.879	1	-.001	10	-5.945e-3	3	744.328	1	NC	1
75		19	max	0	15	.329	3	.007	3	7.886e-3	1	NC	1	NC	1
76			min	0	1	-.605	1	-.004	2	-5.039e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.337	3	.006	3	4.227e-3	3	NC	1	NC	1
78			min	0	1	-.605	1	-.003	2	-8.04e-3	1	NC	1	NC	1
79		2	max	0	15	.492	3	.016	1	4.979e-3	3	NC	5	NC	1
80			min	0	1	-.901	1	-.001	10	-9.331e-3	1	687.26	1	NC	1
81		3	max	0	15	.631	3	.045	1	5.731e-3	3	NC	5	NC	2
82			min	0	1	-1.164	1	.001	10	-1.062e-2	1	364.769	1	4646.573	1
83		4	max	0	15	.743	3	.071	1	6.484e-3	3	NC	15	NC	3
84			min	0	1	-1.368	1	.003	15	-1.191e-2	1	267.335	1	2906.831	1
85		5	max	0	15	.822	3	.086	1	7.236e-3	3	9887.908	15	NC	3
86			min	0	1	-1.5	1	.003	15	-1.321e-2	1	227.868	1	2406.543	1
87		6	max	0	15	.867	3	.084	1	7.989e-3	3	9221.875	15	NC	3
88			min	0	1	-1.558	1	.003	10	-1.45e-2	1	213.874	1	2457.87	1
89		7	max	0	15	.88	3	.066	1	8.741e-3	3	9202.42	15	NC	2
90			min	0	1	-1.553	1	0	10	-1.579e-2	1	215.183	1	3128.845	1
91		8	max	0	15	.871	3	.037	1	9.494e-3	3	9618.129	15	NC	2
92			min	0	1	-1.503	1	-.004	10	-1.708e-2	1	227.122	1	5557.585	1
93		9	max	0	15	.852	3	.019	3	1.025e-2	3	NC	15	NC	1
94			min	0	1	-1.44	1	-.007	2	-1.837e-2	1	244.078	1	NC	1
95		10	max	0	1	.841	3	.019	3	1.1e-2	3	NC	15	NC	1
96			min	0	1	-1.408	1	-.013	2	-1.967e-2	1	253.904	1	NC	1
97		11	max	0	1	.852	3	.019	3	1.025e-2	3	NC	15	NC	1





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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.44	1	-.007	2	-1.837e-2	1	244.078	1	NC	1
99		max	0	1	.871	3	.037	1	9.494e-3	3	9618.129	15	NC	2
100		min	0	15	-1.503	1	-.004	10	-1.708e-2	1	227.122	1	5557.585	1
101		max	0	1	.88	3	.066	1	8.741e-3	3	9202.42	15	NC	2
102		min	0	15	-1.553	1	0	10	-1.579e-2	1	215.183	1	3128.845	1
103		max	0	1	.867	3	.084	1	7.989e-3	3	9221.875	15	NC	3
104		min	0	15	-1.558	1	.003	10	-1.45e-2	1	213.874	1	2457.87	1
105		max	0	1	.822	3	.086	1	7.236e-3	3	9887.908	15	NC	3
106		min	0	15	-1.5	1	.003	15	-1.321e-2	1	227.868	1	2406.543	1
107		max	0	1	.743	3	.071	1	6.484e-3	3	NC	15	NC	3
108		min	0	15	-1.368	1	.003	15	-1.191e-2	1	267.335	1	2906.831	1
109		max	0	1	.631	3	.045	1	5.731e-3	3	NC	5	NC	2
110		min	0	15	-1.164	1	.001	10	-1.062e-2	1	364.769	1	4646.573	1
111		max	0	1	.492	3	.016	1	4.979e-3	3	NC	5	NC	1
112		min	0	15	-.901	1	-.001	10	-9.331e-3	1	687.26	1	NC	1
113		max	0	1	.337	3	.006	3	4.227e-3	3	NC	1	NC	1
114		min	0	15	-.605	1	-.003	2	-8.04e-3	1	NC	1	NC	1
115	M16	max	0	15	.191	1	.005	3	7.876e-3	3	NC	1	NC	1
116		min	0	1	-.118	3	-.003	2	-1.242e-2	1	NC	1	NC	1
117		max	0	15	.053	1	.025	1	8.792e-3	3	NC	5	NC	2
118		min	0	1	-.069	3	0	10	-1.339e-2	1	1406.245	2	8412.541	1
119		max	0	15	.004	13	.058	1	9.709e-3	3	NC	5	NC	3
120		min	0	1	-.089	2	.002	15	-1.436e-2	1	787.221	2	3568.36	1
121		max	0	15	0	15	.085	1	1.063e-2	3	NC	5	NC	3
122		min	0	1	-.151	2	.003	15	-1.532e-2	1	634.406	2	2403.532	1
123		max	0	15	0	13	.099	1	1.154e-2	3	NC	5	NC	3
124		min	0	1	-.153	2	.004	15	-1.629e-2	1	631.668	2	2071.333	1
125		max	0	15	.006	4	.095	1	1.246e-2	3	NC	5	NC	3
126		min	0	1	-.095	2	.004	15	-1.726e-2	1	769.16	2	2167.232	1
127		max	0	15	.055	1	.074	1	1.337e-2	3	NC	3	NC	3
128		min	0	1	-.114	3	.001	10	-1.823e-2	1	1262.749	2	2797.271	1
129		max	0	15	.181	1	.042	1	1.429e-2	3	NC	1	NC	2
130		min	0	1	-.174	3	-.002	10	-1.92e-2	1	3626.767	3	4973.793	1
131		max	0	15	.292	1	.017	3	1.521e-2	3	NC	5	NC	1
132		min	0	1	-.226	3	-.006	10	-2.016e-2	1	1894.589	3	NC	1
133		max	0	1	.342	1	.016	3	1.612e-2	3	NC	5	NC	1
134		min	0	1	-.248	3	-.012	2	-2.113e-2	1	1356.764	1	NC	1
135		max	0	1	.292	1	.017	3	1.521e-2	3	NC	5	NC	1
136		min	0	15	-.226	3	-.006	10	-2.016e-2	1	1894.589	3	NC	1
137		max	0	1	.181	1	.042	1	1.429e-2	3	NC	1	NC	2
138		min	0	15	-.174	3	-.002	10	-1.92e-2	1	3626.767	3	4973.793	1
139		max	0	1	.055	1	.074	1	1.337e-2	3	NC	3	NC	3
140		min	0	15	-.114	3	.001	10	-1.823e-2	1	1262.749	2	2797.271	1
141		max	0	1	.006	4	.095	1	1.246e-2	3	NC	5	NC	3
142		min	0	15	-.095	2	.004	15	-1.726e-2	1	769.16	2	2167.232	1
143		max	0	1	0	13	.099	1	1.154e-2	3	NC	5	NC	3
144		min	0	15	-.153	2	.004	15	-1.629e-2	1	631.668	2	2071.333	1
145		max	0	1	0	15	.085	1	1.063e-2	3	NC	5	NC	3
146		min	0	15	-.151	2	.003	15	-1.532e-2	1	634.406	2	2403.532	1
147		max	0	1	.004	13	.058	1	9.709e-3	3	NC	5	NC	3
148		min	0	15	-.089	2	.002	15	-1.436e-2	1	787.221	2	3568.36	1
149		max	0	1	.053	1	.025	1	8.792e-3	3	NC	5	NC	2
150		min	0	15	-.069	3	0	10	-1.339e-2	1	1406.245	2	8412.541	1
151		max	0	1	.191	1	.005	3	7.876e-3	3	NC	1	NC	1
152		min	0	15	-.118	3	-.003	2	-1.242e-2	1	NC	1	NC	1
153	M2	max	.006	1	.006	2	.009	1	-7.038e-6	15	NC	1	NC	2
154		min	-.007	3	-.01	3	0	15	-1.955e-4	1	9741.88	2	6903.495	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	1	.005	2	.008	1	-6.593e-6	15	NC	1	NC	2
156		min	-.006	3	-.01	3	0	15	-1.832e-4	1	NC	1	7527.133	1
157	3	max	.006	1	.004	2	.007	1	-6.149e-6	15	NC	1	NC	2
158		min	-.006	3	-.01	3	0	15	-1.708e-4	1	NC	1	8269.814	1
159	4	max	.005	1	.004	2	.007	1	-5.705e-6	15	NC	1	NC	2
160		min	-.005	3	-.009	3	0	15	-1.584e-4	1	NC	1	9162.952	1
161	5	max	.005	1	.003	2	.006	1	-5.261e-6	15	NC	1	NC	1
162		min	-.005	3	-.009	3	0	15	-1.461e-4	1	NC	1	NC	1
163	6	max	.005	1	.002	2	.005	1	-4.817e-6	15	NC	1	NC	1
164		min	-.005	3	-.009	3	0	15	-1.337e-4	1	NC	1	NC	1
165	7	max	.004	1	.001	2	.005	1	-4.372e-6	15	NC	1	NC	1
166		min	-.004	3	-.008	3	0	15	-1.213e-4	1	NC	1	NC	1
167	8	max	.004	1	0	2	.004	1	-3.928e-6	15	NC	1	NC	1
168		min	-.004	3	-.008	3	0	15	-1.09e-4	1	NC	1	NC	1
169	9	max	.004	1	0	2	.003	1	-3.484e-6	15	NC	1	NC	1
170		min	-.004	3	-.007	3	0	15	-9.662e-5	1	NC	1	NC	1
171	10	max	.003	1	0	2	.003	1	-3.04e-6	15	NC	1	NC	1
172		min	-.003	3	-.007	3	0	15	-8.426e-5	1	NC	1	NC	1
173	11	max	.003	1	-.001	15	.002	1	-2.595e-6	15	NC	1	NC	1
174		min	-.003	3	-.006	3	0	15	-7.189e-5	1	NC	1	NC	1
175	12	max	.003	1	-.001	15	.002	1	-2.151e-6	15	NC	1	NC	1
176		min	-.003	3	-.006	3	0	15	-5.953e-5	1	NC	1	NC	1
177	13	max	.002	1	-.001	15	.001	1	-1.707e-6	15	NC	1	NC	1
178		min	-.002	3	-.005	3	0	15	-4.717e-5	1	NC	1	NC	1
179	14	max	.002	1	0	15	0	1	-1.263e-6	15	NC	1	NC	1
180		min	-.002	3	-.005	3	0	15	-3.481e-5	1	NC	1	NC	1
181	15	max	.001	1	0	15	0	1	-8.186e-7	15	NC	1	NC	1
182		min	-.001	3	-.004	3	0	15	-2.244e-5	1	NC	1	NC	1
183	16	max	.001	1	0	15	0	1	-3.744e-7	15	NC	1	NC	1
184		min	-.001	3	-.003	4	0	15	-1.008e-5	1	NC	1	NC	1
185	17	max	0	1	0	15	0	1	2.283e-6	1	NC	1	NC	1
186		min	0	3	-.002	4	0	15	-5.059e-7	3	NC	1	NC	1
187	18	max	0	1	0	15	0	1	1.465e-5	1	NC	1	NC	1
188		min	0	3	-.001	4	0	15	4.196e-7	12	NC	1	NC	1
189	19	max	0	1	0	1	0	1	2.701e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	9.583e-7	15	NC	1	NC	1
191	M3	1	max	0	0	1	0	1	-2.946e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-8.279e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	1.692e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	6.083e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	4.212e-5	1	NC	1	NC	1
196		min	0	2	-.005	4	0	15	1.511e-6	15	NC	1	NC	1
197	4	max	0	3	-.002	15	0	1	6.733e-5	1	NC	1	NC	1
198		min	0	2	-.008	4	0	15	2.414e-6	15	NC	1	NC	1
199	5	max	.001	3	-.003	15	0	1	9.253e-5	1	NC	1	NC	1
200		min	0	2	-.011	4	0	15	3.317e-6	15	9254.207	4	NC	1
201	6	max	.002	3	-.003	15	0	1	1.177e-4	1	NC	1	NC	1
202		min	-.001	2	-.014	4	0	15	4.22e-6	15	7429.027	4	NC	1
203	7	max	.002	3	-.004	15	.001	1	1.429e-4	1	NC	5	NC	1
204		min	-.001	2	-.016	4	0	15	5.123e-6	15	6333.795	4	NC	1
205	8	max	.002	3	-.004	15	.001	1	1.681e-4	1	NC	5	NC	1
206		min	-.002	2	-.018	4	0	15	6.026e-6	15	5657.705	4	NC	1
207	9	max	.003	3	-.005	15	.002	1	1.933e-4	1	NC	5	NC	1
208		min	-.002	2	-.02	4	0	15	6.929e-6	15	5254.798	4	NC	1
209	10	max	.003	3	-.005	15	.002	1	2.185e-4	1	NC	5	NC	1
210		min	-.002	2	-.021	4	0	15	7.832e-6	15	5053.925	4	NC	1
211	11	max	.003	3	-.005	15	.003	1	2.437e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.002	2	-.021	4	0	15	8.735e-6	15	5025.143	4	NC	1
213		max	.004	3	-.005	15	.003	1	2.689e-4	1	NC	5	NC	1
214		min	-.003	2	-.02	4	0	15	9.638e-6	15	5168.175	4	NC	1
215		max	.004	3	-.004	15	.003	1	2.941e-4	1	NC	5	NC	1
216		min	-.003	2	-.019	4	0	15	1.054e-5	15	5513.736	4	NC	1
217		max	.004	3	-.004	15	.004	1	3.193e-4	1	NC	5	NC	1
218		min	-.003	2	-.017	4	0	15	1.144e-5	15	6139.772	4	NC	1
219		max	.005	3	-.003	15	.005	1	3.445e-4	1	NC	3	NC	1
220		min	-.003	2	-.015	4	0	15	1.235e-5	15	7220.29	4	NC	1
221		max	.005	3	-.003	15	.005	1	3.697e-4	1	NC	1	NC	1
222		min	-.004	2	-.012	4	0	15	1.325e-5	15	9177.667	4	NC	1
223		max	.005	3	-.002	15	.006	1	3.949e-4	1	NC	1	NC	1
224		min	-.004	2	-.008	4	0	15	1.415e-5	15	NC	1	NC	1
225		max	.006	3	-.001	15	.007	1	4.201e-4	1	NC	1	NC	1
226		min	-.004	2	-.005	1	0	15	1.506e-5	15	NC	1	NC	1
227		max	.006	3	0	15	.008	1	4.453e-4	1	NC	1	NC	1
228		min	-.004	2	-.003	1	0	15	1.596e-5	15	NC	1	NC	1
229	M4	max	.003	1	.004	2	0	15	6.996e-5	1	NC	1	NC	3
230		min	0	3	-.006	3	-.008	1	2.529e-6	15	NC	1	3020.228	1
231		max	.003	1	.004	2	0	15	6.996e-5	1	NC	1	NC	3
232		min	0	3	-.006	3	-.008	1	2.529e-6	15	NC	1	3284.843	1
233		max	.002	1	.004	2	0	15	6.996e-5	1	NC	1	NC	3
234		min	0	3	-.005	3	-.007	1	2.529e-6	15	NC	1	3599.743	1
235		max	.002	1	.003	2	0	15	6.996e-5	1	NC	1	NC	2
236		min	0	3	-.005	3	-.006	1	2.529e-6	15	NC	1	3977.997	1
237		max	.002	1	.003	2	0	15	6.996e-5	1	NC	1	NC	2
238		min	0	3	-.005	3	-.006	1	2.529e-6	15	NC	1	4437.364	1
239		max	.002	1	.003	2	0	15	6.996e-5	1	NC	1	NC	2
240		min	0	3	-.004	3	-.005	1	2.529e-6	15	NC	1	5002.474	1
241		max	.002	1	.003	2	0	15	6.996e-5	1	NC	1	NC	2
242		min	0	3	-.004	3	-.004	1	2.529e-6	15	NC	1	5708.289	1
243		max	.002	1	.002	2	0	15	6.996e-5	1	NC	1	NC	2
244		min	0	3	-.004	3	-.004	1	2.529e-6	15	NC	1	6605.796	1
245		max	.002	1	.002	2	0	15	6.996e-5	1	NC	1	NC	2
246		min	0	3	-.003	3	-.003	1	2.529e-6	15	NC	1	7771.76	1
247		max	.001	1	.002	2	0	15	6.996e-5	1	NC	1	NC	2
248		min	0	3	-.003	3	-.003	1	2.529e-6	15	NC	1	9326.18	1
249		max	.001	1	.002	2	0	15	6.996e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.002	1	2.529e-6	15	NC	1	NC	1
251		max	.001	1	.002	2	0	15	6.996e-5	1	NC	1	NC	1
252		min	0	3	-.002	3	-.002	1	2.529e-6	15	NC	1	NC	1
253		max	0	1	.001	2	0	15	6.996e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	-.001	1	2.529e-6	15	NC	1	NC	1
255		max	0	1	.001	2	0	15	6.996e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	2.529e-6	15	NC	1	NC	1
257		max	0	1	0	2	0	15	6.996e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	2.529e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	6.996e-5	1	NC	1	NC	1
260		min	0	3	0	3	0	1	2.529e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.996e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	2.529e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.996e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.529e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.996e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.529e-6	15	NC	1	NC	1
267	M6	max	.02	1	.023	2	0	1	0	1	NC	3	NC	1
268		min	-.021	3	-.031	3	0	1	0	1	2665.567	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.019	1	.021	2	0	1	0	1	NC	3	NC	1
270		min	-.02	3	-.03	3	0	1	0	1	2924.788	2	NC	1
271	3	max	.018	1	.019	2	0	1	0	1	NC	3	NC	1
272		min	-.019	3	-.028	3	0	1	0	1	3237.415	2	NC	1
273	4	max	.017	1	.017	2	0	1	0	1	NC	3	NC	1
274		min	-.017	3	-.026	3	0	1	0	1	3618.769	2	NC	1
275	5	max	.015	1	.015	2	0	1	0	1	NC	3	NC	1
276		min	-.016	3	-.025	3	0	1	0	1	4090.266	2	NC	1
277	6	max	.014	1	.013	2	0	1	0	1	NC	3	NC	1
278		min	-.015	3	-.023	3	0	1	0	1	4682.594	2	NC	1
279	7	max	.013	1	.011	2	0	1	0	1	NC	3	NC	1
280		min	-.014	3	-.021	3	0	1	0	1	5441.005	2	NC	1
281	8	max	.012	1	.009	2	0	1	0	1	NC	1	NC	1
282		min	-.013	3	-.02	3	0	1	0	1	6434.556	2	NC	1
283	9	max	.011	1	.008	2	0	1	0	1	NC	1	NC	1
284		min	-.012	3	-.018	3	0	1	0	1	7773.029	2	NC	1
285	10	max	.01	1	.006	2	0	1	0	1	NC	1	NC	1
286		min	-.01	3	-.016	3	0	1	0	1	9639.902	2	NC	1
287	11	max	.009	1	.005	2	0	1	0	1	NC	1	NC	1
288		min	-.009	3	-.014	3	0	1	0	1	NC	1	NC	1
289	12	max	.008	1	.004	2	0	1	0	1	NC	1	NC	1
290		min	-.008	3	-.013	3	0	1	0	1	NC	1	NC	1
291	13	max	.007	1	.003	2	0	1	0	1	NC	1	NC	1
292		min	-.007	3	-.011	3	0	1	0	1	NC	1	NC	1
293	14	max	.006	1	.002	2	0	1	0	1	NC	1	NC	1
294		min	-.006	3	-.009	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.005	3	-.007	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.003	3	-.006	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.002	3	-.004	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.009	3	0	1	0	1	NC	1	NC	1
313	5	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.012	3	0	1	0	1	9047.795	3	NC	1
315	6	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.014	3	0	1	0	1	7579.519	3	NC	1
317	7	max	.006	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.005	2	-.016	4	0	1	0	1	6482.001	4	NC	1
319	8	max	.007	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.006	2	-.018	4	0	1	0	1	5779.738	4	NC	1
321	9	max	.008	3	-.005	15	0	1	0	1	NC	2	NC	1
322		min	-.007	2	-.02	4	0	1	0	1	5360.244	4	NC	1
323	10	max	.009	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.008	2	-.021	4	0	1	0	1	5149.039	4	NC	1
325	11	max	.01	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.009	2	-.021	4	0	1	0	1	5114.485	4	NC	1
327		12	max	.011	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.01	2	-.02	4	0	1	0	1	5255.558	4	NC	1
329		13	max	.012	3	-.004	15	0	1	0	1	NC	5	NC	1
330			min	-.011	2	-.019	4	0	1	0	1	5602.953	4	NC	1
331		14	max	.013	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.012	2	-.017	4	0	1	0	1	6235.426	4	NC	1
333		15	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1
334			min	-.013	2	-.015	4	0	1	0	1	7329.25	4	NC	1
335		16	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.014	2	-.012	4	0	1	0	1	9312.65	4	NC	1
337		17	max	.016	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.015	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.017	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.016	2	-.008	1	0	1	0	1	NC	1	NC	1
341		19	max	.018	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.016	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.015	2	0	1	0	1	NC	1	NC	1
344			min	-.002	3	-.018	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.014	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.017	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	1	.014	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.016	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.015	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.014	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
354			min	-.001	3	-.013	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
356			min	-.001	3	-.012	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.009	2	0	1	0	1	NC	1	NC	1
358			min	-.001	3	-.011	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.007	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	1	.006	2	0	15	1.955e-4	1	NC	1	NC	2
382			min	-.007	3	-.01	3	-.009	1	7.038e-6	15	9741.88	2	6903.495	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.006	1	.005	2	0	15	1.832e-4	1	NC	1	NC	2
384		min	-.006	3	-.01	3	-.008	1	6.593e-6	15	NC	1	7527.133	1
385	3	max	.006	1	.004	2	0	15	1.708e-4	1	NC	1	NC	2
386		min	-.006	3	-.01	3	-.007	1	6.149e-6	15	NC	1	8269.814	1
387	4	max	.005	1	.004	2	0	15	1.584e-4	1	NC	1	NC	2
388		min	-.005	3	-.009	3	-.007	1	5.705e-6	15	NC	1	9162.952	1
389	5	max	.005	1	.003	2	0	15	1.461e-4	1	NC	1	NC	1
390		min	-.005	3	-.009	3	-.006	1	5.261e-6	15	NC	1	NC	1
391	6	max	.005	1	.002	2	0	15	1.337e-4	1	NC	1	NC	1
392		min	-.005	3	-.009	3	-.005	1	4.817e-6	15	NC	1	NC	1
393	7	max	.004	1	.001	2	0	15	1.213e-4	1	NC	1	NC	1
394		min	-.004	3	-.008	3	-.005	1	4.372e-6	15	NC	1	NC	1
395	8	max	.004	1	0	2	0	15	1.09e-4	1	NC	1	NC	1
396		min	-.004	3	-.008	3	-.004	1	3.928e-6	15	NC	1	NC	1
397	9	max	.004	1	0	2	0	15	9.662e-5	1	NC	1	NC	1
398		min	-.004	3	-.007	3	-.003	1	3.484e-6	15	NC	1	NC	1
399	10	max	.003	1	0	2	0	15	8.426e-5	1	NC	1	NC	1
400		min	-.003	3	-.007	3	-.003	1	3.04e-6	15	NC	1	NC	1
401	11	max	.003	1	-.001	15	0	15	7.189e-5	1	NC	1	NC	1
402		min	-.003	3	-.006	3	-.002	1	2.595e-6	15	NC	1	NC	1
403	12	max	.003	1	-.001	15	0	15	5.953e-5	1	NC	1	NC	1
404		min	-.003	3	-.006	3	-.002	1	2.151e-6	15	NC	1	NC	1
405	13	max	.002	1	-.001	15	0	15	4.717e-5	1	NC	1	NC	1
406		min	-.002	3	-.005	3	-.001	1	1.707e-6	15	NC	1	NC	1
407	14	max	.002	1	0	15	0	15	3.481e-5	1	NC	1	NC	1
408		min	-.002	3	-.005	3	0	1	1.263e-6	15	NC	1	NC	1
409	15	max	.001	1	0	15	0	15	2.244e-5	1	NC	1	NC	1
410		min	-.001	3	-.004	3	0	1	8.186e-7	15	NC	1	NC	1
411	16	max	.001	1	0	15	0	15	1.008e-5	1	NC	1	NC	1
412		min	-.001	3	-.003	4	0	1	3.744e-7	15	NC	1	NC	1
413	17	max	0	1	0	15	0	15	5.059e-7	3	NC	1	NC	1
414		min	0	3	-.002	4	0	1	-2.283e-6	1	NC	1	NC	1
415	18	max	0	1	0	15	0	15	-4.196e-7	12	NC	1	NC	1
416		min	0	3	-.001	4	0	1	-1.465e-5	1	NC	1	NC	1
417	19	max	0	1	0	1	0	1	-9.583e-7	15	NC	1	NC	1
418		min	0	1	0	1	0	1	-2.701e-5	1	NC	1	NC	1
419	M11	1	max	0	0	1	0	1	8.279e-6	1	NC	1	NC	1
420		min	0	1	0	1	0	1	2.946e-7	15	NC	1	NC	1
421	2	max	0	3	0	15	0	15	-6.083e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	1	-1.692e-5	1	NC	1	NC	1
423	3	max	0	3	-.001	15	0	15	-1.511e-6	15	NC	1	NC	1
424		min	0	2	-.005	4	0	1	-4.212e-5	1	NC	1	NC	1
425	4	max	0	3	-.002	15	0	15	-2.414e-6	15	NC	1	NC	1
426		min	0	2	-.008	4	0	1	-6.733e-5	1	NC	1	NC	1
427	5	max	.001	3	-.003	15	0	15	-3.317e-6	15	NC	1	NC	1
428		min	0	2	-.011	4	0	1	-9.253e-5	1	9254.207	4	NC	1
429	6	max	.002	3	-.003	15	0	15	-4.22e-6	15	NC	1	NC	1
430		min	-.001	2	-.014	4	0	1	-1.177e-4	1	7429.027	4	NC	1
431	7	max	.002	3	-.004	15	0	15	-5.123e-6	15	NC	5	NC	1
432		min	-.001	2	-.016	4	-.001	1	-1.429e-4	1	6333.795	4	NC	1
433	8	max	.002	3	-.004	15	0	15	-6.026e-6	15	NC	5	NC	1
434		min	-.002	2	-.018	4	-.001	1	-1.681e-4	1	5657.705	4	NC	1
435	9	max	.003	3	-.005	15	0	15	-6.929e-6	15	NC	5	NC	1
436		min	-.002	2	-.02	4	-.002	1	-1.933e-4	1	5254.798	4	NC	1
437	10	max	.003	3	-.005	15	0	15	-7.832e-6	15	NC	5	NC	1
438		min	-.002	2	-.021	4	-.002	1	-2.185e-4	1	5053.925	4	NC	1
439	11	max	.003	3	-.005	15	0	15	-8.735e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.002	2	-.021	4	-.003	1	-2.437e-4	1	5025.143	4	NC	1
441		max	.004	3	-.005	15	0	15	-9.638e-6	15	NC	5	NC	1
442		min	-.003	2	-.02	4	-.003	1	-2.689e-4	1	5168.175	4	NC	1
443		max	.004	3	-.004	15	0	15	-1.054e-5	15	NC	5	NC	1
444		min	-.003	2	-.019	4	-.003	1	-2.941e-4	1	5513.736	4	NC	1
445		max	.004	3	-.004	15	0	15	-1.144e-5	15	NC	5	NC	1
446		min	-.003	2	-.017	4	-.004	1	-3.193e-4	1	6139.772	4	NC	1
447		max	.005	3	-.003	15	0	15	-1.235e-5	15	NC	3	NC	1
448		min	-.003	2	-.015	4	-.005	1	-3.445e-4	1	7220.29	4	NC	1
449		max	.005	3	-.003	15	0	15	-1.325e-5	15	NC	1	NC	1
450		min	-.004	2	-.012	4	-.005	1	-3.697e-4	1	9177.667	4	NC	1
451		max	.005	3	-.002	15	0	15	-1.415e-5	15	NC	1	NC	1
452		min	-.004	2	-.008	4	-.006	1	-3.949e-4	1	NC	1	NC	1
453		max	.006	3	-.001	15	0	15	-1.506e-5	15	NC	1	NC	1
454		min	-.004	2	-.005	1	-.007	1	-4.201e-4	1	NC	1	NC	1
455		max	.006	3	0	15	0	15	-1.596e-5	15	NC	1	NC	1
456		min	-.004	2	-.003	1	-.008	1	-4.453e-4	1	NC	1	NC	1
457	M12	max	.003	1	.004	2	.008	1	-2.529e-6	15	NC	1	NC	3
458		min	0	3	-.006	3	0	15	-6.996e-5	1	NC	1	3020.228	1
459		max	.003	1	.004	2	.008	1	-2.529e-6	15	NC	1	NC	3
460		min	0	3	-.006	3	0	15	-6.996e-5	1	NC	1	3284.843	1
461		max	.002	1	.004	2	.007	1	-2.529e-6	15	NC	1	NC	3
462		min	0	3	-.005	3	0	15	-6.996e-5	1	NC	1	3599.743	1
463		max	.002	1	.003	2	.006	1	-2.529e-6	15	NC	1	NC	2
464		min	0	3	-.005	3	0	15	-6.996e-5	1	NC	1	3977.997	1
465		max	.002	1	.003	2	.006	1	-2.529e-6	15	NC	1	NC	2
466		min	0	3	-.005	3	0	15	-6.996e-5	1	NC	1	4437.364	1
467		max	.002	1	.003	2	.005	1	-2.529e-6	15	NC	1	NC	2
468		min	0	3	-.004	3	0	15	-6.996e-5	1	NC	1	5002.474	1
469		max	.002	1	.003	2	.004	1	-2.529e-6	15	NC	1	NC	2
470		min	0	3	-.004	3	0	15	-6.996e-5	1	NC	1	5708.289	1
471		max	.002	1	.002	2	.004	1	-2.529e-6	15	NC	1	NC	2
472		min	0	3	-.004	3	0	15	-6.996e-5	1	NC	1	6605.796	1
473		max	.002	1	.002	2	.003	1	-2.529e-6	15	NC	1	NC	2
474		min	0	3	-.003	3	0	15	-6.996e-5	1	NC	1	7771.76	1
475		max	.001	1	.002	2	.003	1	-2.529e-6	15	NC	1	NC	2
476		min	0	3	-.003	3	0	15	-6.996e-5	1	NC	1	9326.18	1
477		max	.001	1	.002	2	.002	1	-2.529e-6	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-6.996e-5	1	NC	1	NC	1
479		max	.001	1	.002	2	.002	1	-2.529e-6	15	NC	1	NC	1
480		min	0	3	-.002	3	0	15	-6.996e-5	1	NC	1	NC	1
481		max	0	1	.001	2	.001	1	-2.529e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-6.996e-5	1	NC	1	NC	1
483		max	0	1	.001	2	0	1	-2.529e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-6.996e-5	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-2.529e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-6.996e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-2.529e-6	15	NC	1	NC	1
488		min	0	3	0	3	0	15	-6.996e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.529e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-6.996e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.529e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.996e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.529e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.996e-5	1	NC	1	NC	1
495	M1	max	.008	3	.199	1	0	1	1.046e-2	1	NC	1	NC	1
496		min	-.004	2	-.042	3	0	15	-1.66e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.008	3	.099	1	0	15	5.051e-3	1	NC	5	NC	1
498			min	-.004	2	-.021	3	-.006	1	-8.24e-3	3	1348.126	1	NC	1
499		3	max	.008	3	.01	3	0	15	1.448e-5	10	NC	5	NC	1
500			min	-.004	2	-.009	2	-.009	1	-1.868e-4	1	648.837	1	NC	1
501		4	max	.007	3	.061	3	0	15	4.42e-3	1	NC	15	NC	1
502			min	-.004	2	-.13	1	-.008	1	-3.7e-3	3	409.282	1	NC	1
503		5	max	.007	3	.124	3	0	15	9.027e-3	1	9816.902	15	NC	1
504			min	-.004	2	-.257	1	-.006	1	-7.312e-3	3	295.033	1	NC	1
505		6	max	.007	3	.192	3	0	15	1.363e-2	1	7771.344	15	NC	1
506			min	-.004	2	-.381	1	-.003	1	-1.092e-2	3	232.117	1	NC	1
507		7	max	.007	3	.258	3	0	1	1.824e-2	1	6562.284	15	NC	1
508			min	-.004	2	-.491	1	0	3	-1.454e-2	3	195.005	1	NC	1
509		8	max	.007	3	.312	3	0	1	2.285e-2	1	5846.242	15	NC	1
510			min	-.004	2	-.579	1	0	15	-1.815e-2	3	173.066	1	NC	1
511		9	max	.007	3	.348	3	0	15	2.509e-2	1	5471.367	15	NC	1
512			min	-.003	2	-.634	1	0	1	-1.853e-2	3	161.631	1	NC	1
513		10	max	.006	3	.361	3	0	1	2.574e-2	1	5356.735	15	NC	1
514			min	-.003	2	-.652	1	0	15	-1.674e-2	3	158.204	1	NC	1
515		11	max	.006	3	.352	3	0	1	2.639e-2	1	5471.147	15	NC	1
516			min	-.003	2	-.634	1	0	15	-1.496e-2	3	161.858	1	NC	1
517		12	max	.006	3	.323	3	0	15	2.483e-2	1	5845.772	15	NC	1
518			min	-.003	2	-.577	1	0	1	-1.286e-2	3	173.749	1	NC	1
519		13	max	.006	3	.275	3	0	15	1.998e-2	1	6561.458	15	NC	1
520			min	-.003	2	-.488	1	0	1	-1.029e-2	3	196.649	1	NC	1
521		14	max	.006	3	.214	3	.002	1	1.512e-2	1	7769.943	15	NC	1
522			min	-.003	2	-.376	1	0	15	-7.722e-3	3	235.593	1	NC	1
523		15	max	.006	3	.145	3	.005	1	1.027e-2	1	9814.484	15	NC	1
524			min	-.003	2	-.251	1	0	15	-5.151e-3	3	302.096	1	NC	1
525		16	max	.006	3	.073	3	.008	1	5.413e-3	1	NC	15	NC	1
526			min	-.003	2	-.124	1	0	15	-2.58e-3	3	423.92	1	NC	1
527		17	max	.005	3	.004	3	.008	1	5.573e-4	1	NC	5	NC	1
528			min	-.003	2	-.005	2	0	15	-8.838e-6	3	681.266	1	NC	1
529		18	max	.005	3	.098	1	.006	1	6.707e-3	2	NC	5	NC	1
530			min	-.003	2	-.059	3	0	15	-2.255e-3	3	1429.674	1	NC	1
531		19	max	.005	3	.191	1	0	15	1.335e-2	2	NC	1	NC	1
532			min	-.003	2	-.118	3	0	1	-4.587e-3	3	NC	1	NC	1
533	M5	1	max	.023	3	.363	1	0	1	0	1	NC	1	NC	1
534			min	-.016	2	-.022	3	0	1	0	1	NC	1	NC	1
535		2	max	.023	3	.182	1	0	1	0	1	NC	5	NC	1
536			min	-.016	2	-.013	3	0	1	0	1	744.901	1	NC	1
537		3	max	.023	3	.031	3	0	1	0	1	NC	15	NC	1
538			min	-.016	2	-.028	2	0	1	0	1	344.966	1	NC	1
539		4	max	.022	3	.138	3	0	1	0	1	7697.116	15	NC	1
540			min	-.015	2	-.288	1	0	1	0	1	207.096	1	NC	1
541		5	max	.022	3	.29	3	0	1	0	1	5353.745	15	NC	1
542			min	-.015	2	-.576	1	0	1	0	1	143.432	1	NC	1
543		6	max	.021	3	.463	3	0	1	0	1	4103.22	15	NC	1
544			min	-.015	2	-.866	1	0	1	0	1	109.551	1	NC	1
545		7	max	.021	3	.632	3	0	1	0	1	3384.316	15	NC	1
546			min	-.014	2	-1.132	1	0	1	0	1	90.113	1	NC	1
547		8	max	.02	3	.776	3	0	1	0	1	2967.87	15	NC	1
548			min	-.014	2	-1.345	1	0	1	0	1	78.87	1	NC	1
549		9	max	.02	3	.868	3	0	1	0	1	2754.634	15	NC	1
550			min	-.014	2	-1.48	1	0	1	0	1	73.124	1	NC	1
551		10	max	.02	3	.902	3	0	1	0	1	2690.377	15	NC	1
552			min	-.014	2	-1.525	1	0	1	0	1	71.417	1	NC	1
553		11	max	.019	3	.88	3	0	1	0	1	2754.746	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.013	2	-1.479	1	0	1	0	1	73.24	1	NC	1
555		12	max	.019	3	.803	3	0	1	0	1	2968.135	15	NC	1
556			min	-.013	2	-1.341	1	0	1	0	1	79.259	1	NC	1
557		13	max	.018	3	.679	3	0	1	0	1	3384.85	15	NC	1
558			min	-.013	2	-1.122	1	0	1	0	1	91.136	1	NC	1
559		14	max	.018	3	.522	3	0	1	0	1	4104.252	15	NC	1
560			min	-.013	2	-.851	1	0	1	0	1	111.888	1	NC	1
561		15	max	.017	3	.348	3	0	1	0	1	5355.774	15	NC	1
562			min	-.012	2	-.556	1	0	1	0	1	148.591	1	NC	1
563		16	max	.017	3	.173	3	0	1	0	1	7701.36	15	NC	1
564			min	-.012	2	-.267	1	0	1	0	1	218.913	1	NC	1
565		17	max	.016	3	.01	3	0	1	0	1	NC	15	NC	1
566			min	-.012	2	-.015	2	0	1	0	1	374.424	1	NC	1
567		18	max	.016	3	.18	1	0	1	0	1	NC	5	NC	1
568			min	-.012	2	-.126	3	0	1	0	1	825.729	1	NC	1
569		19	max	.016	3	.342	1	0	1	0	1	NC	1	NC	1
570			min	-.012	2	-.248	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.008	3	.199	1	0	15	1.66e-2	3	NC	1	NC	1
572			min	-.004	2	-.042	3	0	1	-1.046e-2	1	NC	1	NC	1
573		2	max	.008	3	.099	1	.006	1	8.24e-3	3	NC	5	NC	1
574			min	-.004	2	-.021	3	0	15	-5.051e-3	1	1348.126	1	NC	1
575		3	max	.008	3	.01	3	.009	1	1.868e-4	1	NC	5	NC	1
576			min	-.004	2	-.009	2	0	15	-1.448e-5	10	648.837	1	NC	1
577		4	max	.007	3	.061	3	.008	1	3.7e-3	3	NC	15	NC	1
578			min	-.004	2	-.13	1	0	15	-4.42e-3	1	409.282	1	NC	1
579		5	max	.007	3	.124	3	.006	1	7.312e-3	3	9816.902	15	NC	1
580			min	-.004	2	-.257	1	0	15	-9.027e-3	1	295.033	1	NC	1
581		6	max	.007	3	.192	3	.003	1	1.092e-2	3	7771.344	15	NC	1
582			min	-.004	2	-.381	1	0	15	-1.363e-2	1	232.117	1	NC	1
583		7	max	.007	3	.258	3	0	3	1.454e-2	3	6562.284	15	NC	1
584			min	-.004	2	-.491	1	0	1	-1.824e-2	1	195.005	1	NC	1
585		8	max	.007	3	.312	3	0	15	1.815e-2	3	5846.242	15	NC	1
586			min	-.004	2	-.579	1	0	1	-2.285e-2	1	173.066	1	NC	1
587		9	max	.007	3	.348	3	0	1	1.853e-2	3	5471.367	15	NC	1
588			min	-.003	2	-.634	1	0	15	-2.509e-2	1	161.631	1	NC	1
589		10	max	.006	3	.361	3	0	15	1.674e-2	3	5356.735	15	NC	1
590			min	-.003	2	-.652	1	0	1	-2.574e-2	1	158.204	1	NC	1
591		11	max	.006	3	.352	3	0	15	1.496e-2	3	5471.147	15	NC	1
592			min	-.003	2	-.634	1	0	1	-2.639e-2	1	161.858	1	NC	1
593		12	max	.006	3	.323	3	0	1	1.286e-2	3	5845.772	15	NC	1
594			min	-.003	2	-.577	1	0	15	-2.483e-2	1	173.749	1	NC	1
595		13	max	.006	3	.275	3	0	1	1.029e-2	3	6561.458	15	NC	1
596			min	-.003	2	-.488	1	0	15	-1.998e-2	1	196.649	1	NC	1
597		14	max	.006	3	.214	3	0	15	7.722e-3	3	7769.943	15	NC	1
598			min	-.003	2	-.376	1	-.002	1	-1.512e-2	1	235.593	1	NC	1
599		15	max	.006	3	.145	3	0	15	5.151e-3	3	9814.484	15	NC	1
600			min	-.003	2	-.251	1	-.005	1	-1.027e-2	1	302.096	1	NC	1
601		16	max	.006	3	.073	3	0	15	2.58e-3	3	NC	15	NC	1
602			min	-.003	2	-.124	1	-.008	1	-5.413e-3	1	423.92	1	NC	1
603		17	max	.005	3	.004	3	0	15	8.838e-6	3	NC	5	NC	1
604			min	-.003	2	-.005	2	-.008	1	-5.573e-4	1	681.266	1	NC	1
605		18	max	.005	3	.098	1	0	15	2.255e-3	3	NC	5	NC	1
606			min	-.003	2	-.059	3	-.006	1	-6.707e-3	2	1429.674	1	NC	1
607		19	max	.005	3	.191	1	0	1	4.587e-3	3	NC	1	NC	1
608			min	-.003	2	-.118	3	0	15	-1.335e-2	2	NC	1	NC	1



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Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

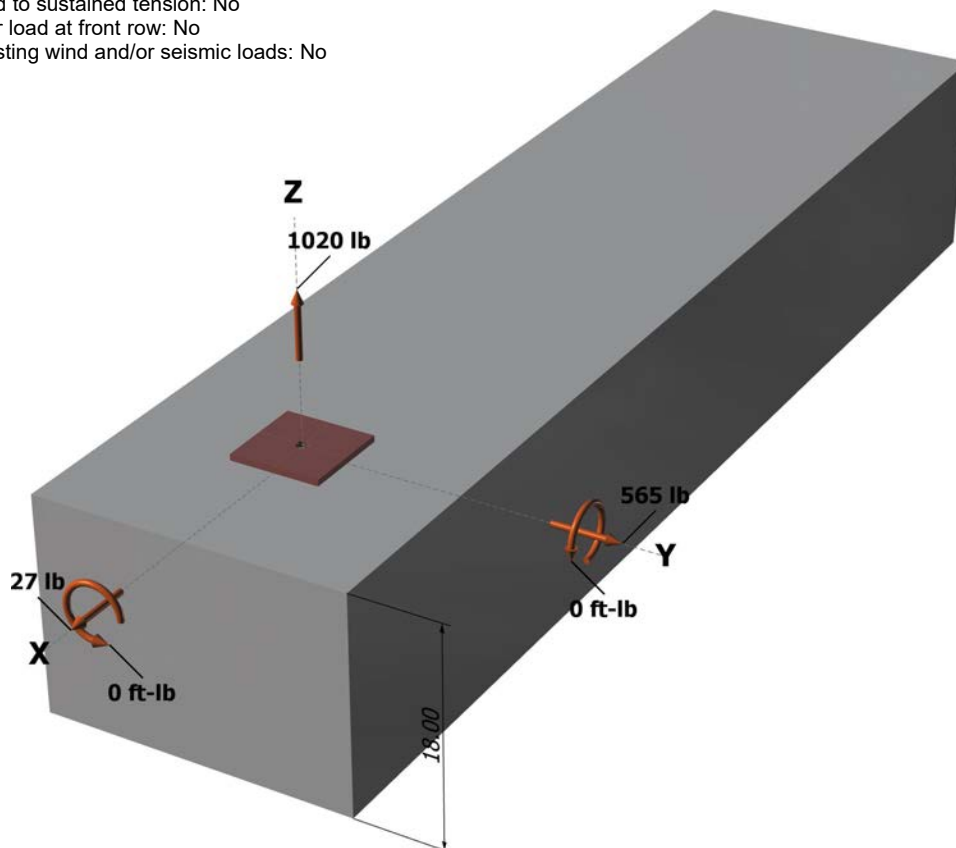
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.6025.0

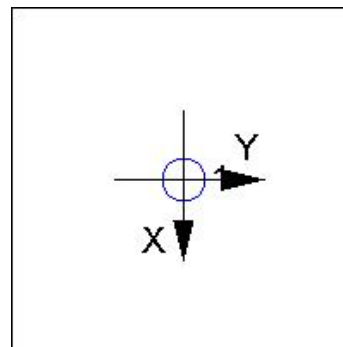
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

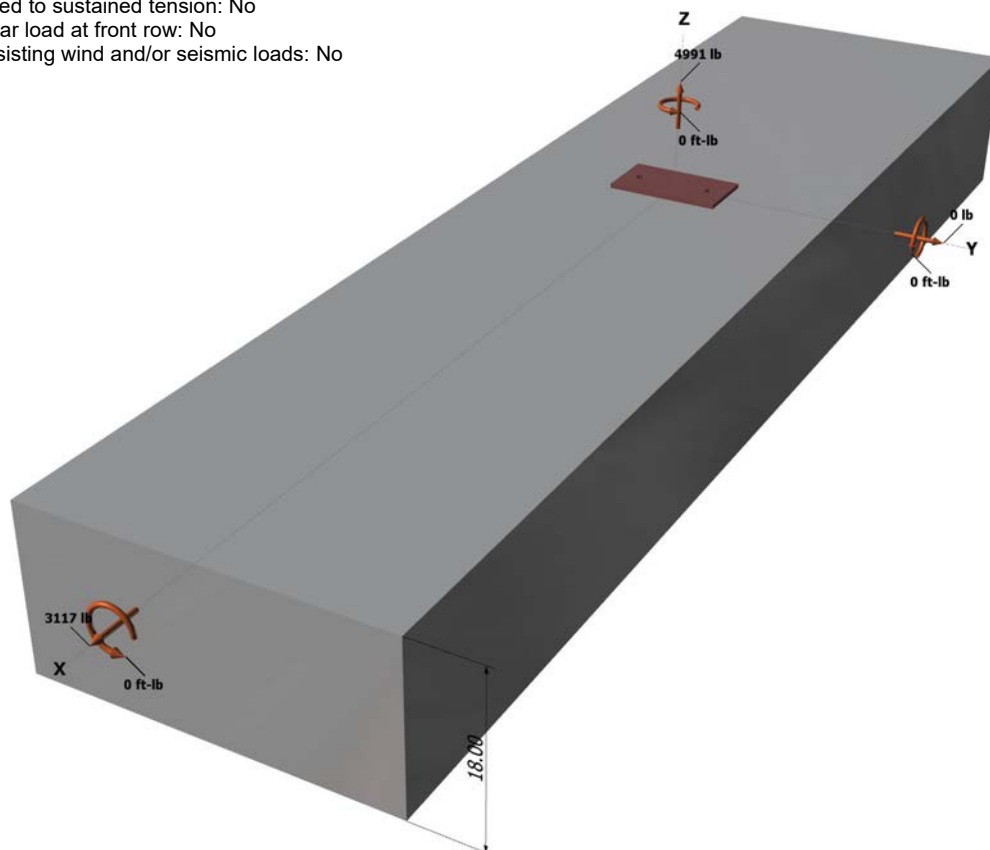
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



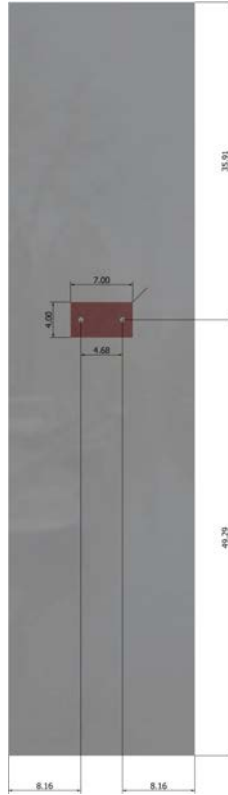
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

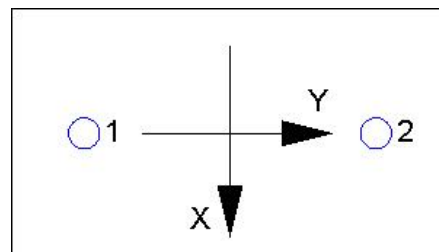
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.