

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 15°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	22.68 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	1.00	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

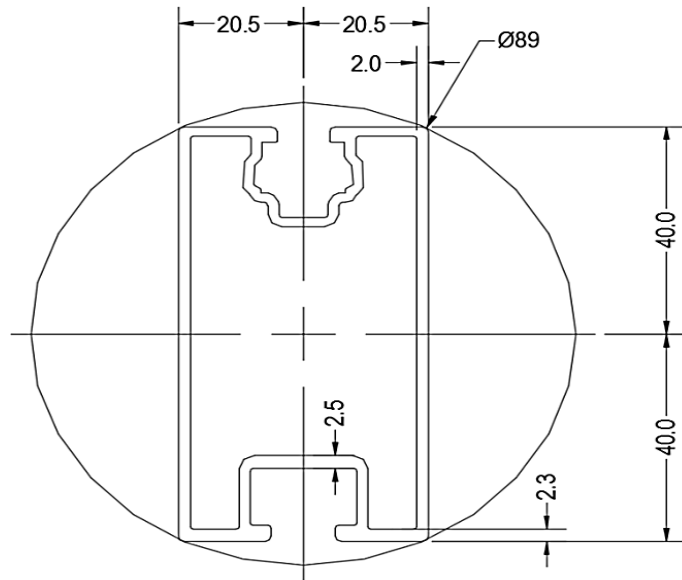
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	102 in
ΦF_{ty} STRONG-AXIS =	28.61 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.327 k-ft
M_z =	0.193 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	98%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

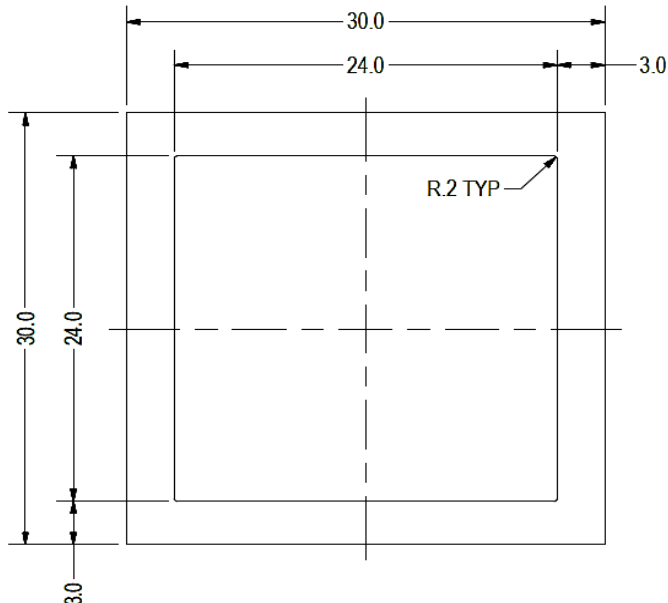
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.93 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.647 k-ft
M_z =	0.000 k-ft
P_n =	0.197 k
$M_{y \text{ allowable}}$ =	1.469 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	46%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.758 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	15%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

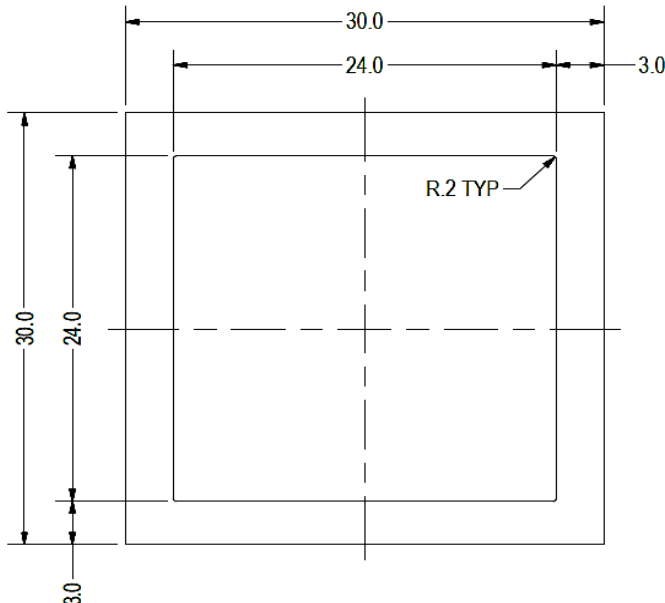
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.140 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	4%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

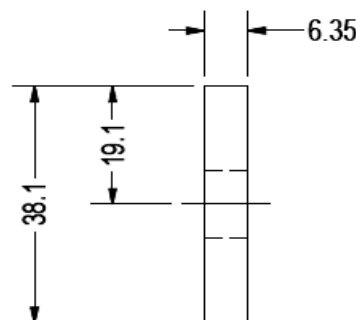
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.422 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.007 k-ft
P_n =	0.032 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	16%



A cross brace kit is required every 12 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

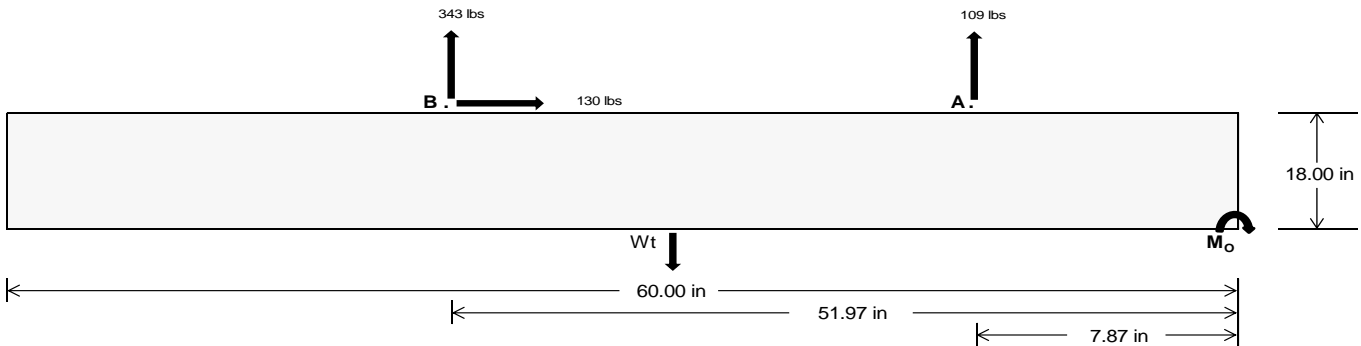
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	460.89	1433.11	k
Compressive Load =	2285.07	1677.08	k
Lateral Load =	4.36	540.61	k
Moment (Weak Axis) =	0.01	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21048.9$ in-lbs
Resisting Force Required = 701.63 lbs
S.F. = 1.67
Weight Required = 1169.39 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 129.90 lbs
Friction = 0.4
Weight Required = 324.74 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 129.90 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	885 lbs	885 lbs	885 lbs	885 lbs	609 lbs	609 lbs	609 lbs	609 lbs	1060 lbs	1060 lbs	1060 lbs	1060 lbs	-219 lbs	-219 lbs	-219 lbs	-219 lbs
F_B	652 lbs	652 lbs	652 lbs	652 lbs	445 lbs	445 lbs	445 lbs	445 lbs	777 lbs	777 lbs	777 lbs	777 lbs	-687 lbs	-687 lbs	-687 lbs	-687 lbs
F_V	62 lbs	62 lbs	62 lbs	62 lbs	233 lbs	233 lbs	233 lbs	233 lbs	218 lbs	218 lbs	218 lbs	218 lbs	-260 lbs	-260 lbs	-260 lbs	-260 lbs
P_{total}	3440 lbs	3531 lbs	3622 lbs	3712 lbs	2957 lbs	3048 lbs	3138 lbs	3229 lbs	3740 lbs	3831 lbs	3921 lbs	4012 lbs	236 lbs	291 lbs	345 lbs	399 lbs
M	532 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	656 lbs-ft	858 lbs-ft	858 lbs-ft	858 lbs-ft	858 lbs-ft	465 lbs-ft	465 lbs-ft	465 lbs-ft	465 lbs-ft
e	0.15 ft	0.15 ft	0.15 ft	0.14 ft	0.22 ft	0.22 ft	0.21 ft	0.20 ft	0.23 ft	0.22 ft	0.22 ft	0.21 ft	1.97 ft	1.60 ft	1.35 ft	1.16 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	320.2 psf	315.5 psf	311.2 psf	307.3 psf	247.9 psf	246.6 psf	245.3 psf	244.1 psf	309.7 psf	305.5 psf	301.7 psf	298.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	466.2 psf	454.9 psf	444.6 psf	435.1 psf	428.0 psf	418.4 psf	409.7 psf	401.7 psf	545.2 psf	530.3 psf	516.7 psf	504.2 psf	168.8 psf	117.3 psf	104.0 psf	99.6 psf

Maximum Bearing Pressure = 545 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

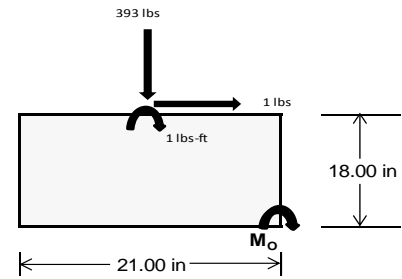
Overturning Check

$M_o = 341.8 \text{ ft-lbs}$
 Resisting Force Required = 390.64 lbs
 S.F. = 1.67
 Weight Required = 651.06 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	93 lbs	263 lbs	88 lbs	398 lbs	1244 lbs	393 lbs	27 lbs	77 lbs	26 lbs
F_v	4 lbs	3 lbs	0 lbs	18 lbs	18 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2449 lbs	2619 lbs	2444 lbs	2641 lbs	3487 lbs	2636 lbs	716 lbs	766 lbs	715 lbs
M	5 lbs-ft	5 lbs-ft	0 lbs-ft	32 lbs-ft	26 lbs-ft	2 lbs-ft	2 lbs-ft	1 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.73 ft	1.73 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	277.8 sqft	297.3 sqft	279.2 sqft	289.5 sqft	388.2 sqft	300.4 sqft	81.2 sqft	86.9 sqft	81.6 sqft
f_{max}	282.0 psf	301.3 psf	279.4 psf	314.2 psf	408.8 psf	302.2 psf	82.5 psf	88.1 psf	81.7 psf



Maximum Bearing Pressure = 409 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.341 k
Allowable Uplift =	1.214 k
Utilization =	<u>28%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.007 k
Allowable Uplift =	1.116 k
Utilization =	<u>90%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.758 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>31%</u>

Diagonal Strut

Maximum Axial Load =	0.151 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



Rear Strut

Maximum Axial Load =	1.422 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

Bracing

Maximum Axial Load =	0.032 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	28.39 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.568 in
Max Drift, Δ_{MAX} =	0.053 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.45 \\
 &20.4426 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.45 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.469 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	91.559	2	358.381	1	.029	2	0	1	0	1	0	1
2		min	-130.415	3	-334.733	3	-.095	1	0	3	0	1	0	1
3	N7	max	0	15	590.713	1	-.054	15	0	15	0	1	0	1
4		min	-.188	1	-100.441	3	-1.5	1	-.003	1	0	1	0	1
5	N15	max	0	15	1757.747	1	.511	1	.001	1	0	1	0	1
6		min	-2.019	1	-354.527	3	-.198	3	0	3	0	1	0	1
7	N16	max	397.285	2	1290.059	1	-.228	10	0	1	0	1	0	1
8		min	-415.855	3	-1102.389	3	-29.803	1	0	3	0	1	0	1
9	N23	max	0	15	590.619	1	3.357	1	.006	1	0	1	0	1
10		min	-.187	1	-100.037	3	.113	15	0	15	0	1	0	1
11	N24	max	91.973	2	363.987	1	27.53	1	.002	1	0	1	0	1
12		min	-130.454	3	-331.635	3	.046	10	0	3	0	1	0	1
13	Totals:	max	579.037	2	4951.507	1	0	1						
14		min	-677.025	3	-2323.763	3	0	10						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	432.104	1	.659	4	.912	1	0	15	0	3	0	1
2			min	-336.211	3	.157	15	-.043	3	-.001	1	0	2	0	1
3		2	max	432.201	1	.622	4	.912	1	0	15	0	1	0	15
4			min	-336.139	3	.148	15	-.043	3	-.001	1	0	10	0	4
5		3	max	432.297	1	.584	4	.912	1	0	15	0	1	0	15
6			min	-336.066	3	.139	15	-.043	3	-.001	1	0	15	0	4
7		4	max	432.393	1	.546	4	.912	1	0	15	0	1	0	15
8			min	-335.994	3	.13	15	-.043	3	-.001	1	0	12	0	4
9		5	max	432.49	1	.508	4	.912	1	0	15	0	1	0	15
10			min	-335.922	3	.121	15	-.043	3	-.001	1	0	12	0	4
11		6	max	432.586	1	.47	4	.912	1	0	15	0	1	0	15
12			min	-335.85	3	.112	15	-.043	3	-.001	1	0	3	0	4
13		7	max	432.683	1	.433	4	.912	1	0	15	0	1	0	15
14			min	-335.777	3	.103	15	-.043	3	-.001	1	0	3	0	4
15		8	max	432.779	1	.395	4	.912	1	0	15	0	1	0	15
16			min	-335.705	3	.095	15	-.043	3	-.001	1	0	3	0	4
17		9	max	432.875	1	.357	4	.912	1	0	15	.001	1	0	15
18			min	-335.633	3	.086	15	-.043	3	-.001	1	0	3	0	4
19		10	max	432.972	1	.319	4	.912	1	0	15	.001	1	0	15
20			min	-335.56	3	.077	15	-.043	3	-.001	1	0	3	0	4
21		11	max	433.068	1	.281	4	.912	1	0	15	.001	1	0	15
22			min	-335.488	3	.068	15	-.043	3	-.001	1	0	3	0	4
23		12	max	433.164	1	.243	4	.912	1	0	15	.001	1	0	15
24			min	-335.416	3	.059	15	-.043	3	-.001	1	0	3	0	4
25		13	max	433.261	1	.206	4	.912	1	0	15	.002	1	0	15
26			min	-335.344	3	.05	15	-.043	3	-.001	1	0	3	0	4
27		14	max	433.357	1	.168	4	.912	1	0	15	.002	1	0	15
28			min	-335.271	3	.041	15	-.043	3	-.001	1	0	3	0	4
29		15	max	433.453	1	.13	4	.912	1	0	15	.002	1	0	15
30			min	-335.199	3	.032	15	-.043	3	-.001	1	0	3	0	4
31		16	max	433.55	1	.092	4	.912	1	0	15	.002	1	0	15
32			min	-335.127	3	.023	15	-.043	3	-.001	1	0	3	0	4
33		17	max	433.646	1	.054	4	.912	1	0	15	.002	1	0	15
34			min	-335.055	3	-.003	1	-.043	3	-.001	1	0	3	0	4
35		18	max	433.743	1	.028	10	.912	1	0	15	.002	1	0	15
36			min	-334.982	3	-.033	1	-.043	3	-.001	1	0	3	0	4
37		19	max	433.839	1	.004	10	.912	1	0	15	.002	1	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-334.91	3	-.062	1	-.043	3	-.001	1	0	3	0	4
39	M3	1	max	32.075	10	1.811	4	-.024	15	0	15	.002	1	0	4
40			min	-132.606	1	.427	15	-.79	1	0	1	0	15	0	15
41		2	max	32.019	10	1.633	4	-.024	15	0	15	.002	1	0	4
42			min	-132.673	1	.385	15	-.79	1	0	1	0	15	0	15
43		3	max	31.963	10	1.455	4	-.024	15	0	15	.002	1	0	10
44			min	-132.74	1	.343	15	-.79	1	0	1	0	15	0	1
45		4	max	31.907	10	1.277	4	-.024	15	0	15	.002	1	0	15
46			min	-132.807	1	.301	15	-.79	1	0	1	0	15	0	1
47		5	max	31.852	10	1.099	4	-.024	15	0	15	.002	1	0	15
48			min	-132.874	1	.259	15	-.79	1	0	1	0	15	0	4
49		6	max	31.796	10	.921	4	-.024	15	0	15	.001	1	0	15
50			min	-132.941	1	.218	15	-.79	1	0	1	0	15	0	4
51		7	max	31.74	10	.743	4	-.024	15	0	15	.001	1	0	15
52			min	-133.008	1	.176	15	-.79	1	0	1	0	15	0	4
53		8	max	31.684	10	.565	4	-.024	15	0	15	.001	1	0	15
54			min	-133.076	1	.134	15	-.79	1	0	1	0	15	0	4
55		9	max	31.628	10	.387	4	-.024	15	0	15	0	1	0	15
56			min	-133.143	1	.092	15	-.79	1	0	1	0	15	-.001	4
57		10	max	31.572	10	.209	4	-.024	15	0	15	0	1	0	15
58			min	-133.21	1	.05	15	-.79	1	0	1	0	15	-.001	4
59		11	max	31.516	10	.031	10	-.024	15	0	15	0	1	0	15
60			min	-133.277	1	-.006	1	-.79	1	0	1	0	12	-.001	4
61		12	max	31.46	10	-.033	15	-.024	15	0	15	0	1	0	15
62			min	-133.344	1	-.147	4	-.79	1	0	1	0	12	-.001	4
63		13	max	31.404	10	-.075	15	-.024	15	0	15	0	1	0	15
64			min	-133.411	1	-.325	4	-.79	1	0	1	0	12	-.001	4
65		14	max	31.348	10	-.117	15	-.024	15	0	15	0	1	0	15
66			min	-133.478	1	-.503	4	-.79	1	0	1	0	3	-.001	4
67		15	max	31.292	10	-.159	15	-.024	15	0	15	0	15	0	15
68			min	-133.545	1	-.681	4	-.79	1	0	1	0	1	0	4
69		16	max	31.237	10	-.201	15	-.024	15	0	15	0	15	0	15
70			min	-133.612	1	-.859	4	-.79	1	0	1	0	1	0	4
71		17	max	31.181	10	-.243	15	-.024	15	0	15	0	15	0	15
72			min	-133.679	1	-1.037	4	-.79	1	0	1	0	1	0	4
73		18	max	31.125	10	-.285	15	-.024	15	0	15	0	15	0	15
74			min	-133.747	1	-1.216	4	-.79	1	0	1	0	1	0	4
75		19	max	31.069	10	-.326	15	-.024	15	0	15	0	15	0	1
76			min	-133.814	1	-1.394	4	-.79	1	0	1	0	1	0	1
77	M4	1	max	589.548	1	0	1	-.054	15	0	1	0	3	0	1
78			min	-101.315	3	0	1	-1.664	1	0	1	0	1	0	1
79		2	max	589.613	1	0	1	-.054	15	0	1	0	12	0	1
80			min	-101.266	3	0	1	-1.664	1	0	1	0	1	0	1
81		3	max	589.678	1	0	1	-.054	15	0	1	0	15	0	1
82			min	-101.218	3	0	1	-1.664	1	0	1	0	1	0	1
83		4	max	589.743	1	0	1	-.054	15	0	1	0	15	0	1
84			min	-101.169	3	0	1	-1.664	1	0	1	0	1	0	1
85		5	max	589.807	1	0	1	-.054	15	0	1	0	15	0	1
86			min	-101.121	3	0	1	-1.664	1	0	1	0	1	0	1
87		6	max	589.872	1	0	1	-.054	15	0	1	0	15	0	1
88			min	-101.072	3	0	1	-1.664	1	0	1	0	1	0	1
89		7	max	589.937	1	0	1	-.054	15	0	1	0	15	0	1
90			min	-101.024	3	0	1	-1.664	1	0	1	0	1	0	1
91		8	max	590.001	1	0	1	-.054	15	0	1	0	15	0	1
92			min	-100.975	3	0	1	-1.664	1	0	1	-.001	1	0	1
93		9	max	590.066	1	0	1	-.054	15	0	1	0	15	0	1
94			min	-100.927	3	0	1	-1.664	1	0	1	-.001	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	590.131	1	0	1	-.054	15	0	1	0	15	0	1
96			min	-100.878	3	0	1	-1.664	1	0	1	-.001	1	0	1
97		11	max	590.195	1	0	1	-.054	15	0	1	0	15	0	1
98			min	-100.83	3	0	1	-1.664	1	0	1	-.002	1	0	1
99		12	max	590.26	1	0	1	-.054	15	0	1	0	15	0	1
100			min	-100.781	3	0	1	-1.664	1	0	1	-.002	1	0	1
101		13	max	590.325	1	0	1	-.054	15	0	1	0	15	0	1
102			min	-100.733	3	0	1	-1.664	1	0	1	-.002	1	0	1
103		14	max	590.39	1	0	1	-.054	15	0	1	0	15	0	1
104			min	-100.684	3	0	1	-1.664	1	0	1	-.002	1	0	1
105		15	max	590.454	1	0	1	-.054	15	0	1	0	15	0	1
106			min	-100.636	3	0	1	-1.664	1	0	1	-.002	1	0	1
107		16	max	590.519	1	0	1	-.054	15	0	1	0	15	0	1
108			min	-100.587	3	0	1	-1.664	1	0	1	-.002	1	0	1
109		17	max	590.584	1	0	1	-.054	15	0	1	0	15	0	1
110			min	-100.539	3	0	1	-1.664	1	0	1	-.002	1	0	1
111		18	max	590.648	1	0	1	-.054	15	0	1	0	15	0	1
112			min	-100.49	3	0	1	-1.664	1	0	1	-.003	1	0	1
113		19	max	590.713	1	0	1	-.054	15	0	1	0	15	0	1
114			min	-100.441	3	0	1	-1.664	1	0	1	-.003	1	0	1
115	M6	1	max	1420.266	1	.642	4	.346	1	0	1	0	3	0	1
116			min	-1104.863	3	.154	15	-.104	3	0	15	0	1	0	1
117		2	max	1420.362	1	.604	4	.346	1	0	1	0	3	0	15
118			min	-1104.791	3	.146	15	-.104	3	0	15	0	2	0	4
119		3	max	1420.459	1	.566	4	.346	1	0	1	0	1	0	15
120			min	-1104.719	3	.137	15	-.104	3	0	15	0	12	0	4
121		4	max	1420.555	1	.528	4	.346	1	0	1	0	1	0	15
122			min	-1104.647	3	.128	15	-.104	3	0	15	0	3	0	4
123		5	max	1420.652	1	.49	4	.346	1	0	1	0	1	0	15
124			min	-1104.574	3	.119	15	-.104	3	0	15	0	3	0	4
125		6	max	1420.748	1	.452	4	.346	1	0	1	0	1	0	15
126			min	-1104.502	3	.11	15	-.104	3	0	15	0	3	0	4
127		7	max	1420.844	1	.415	4	.346	1	0	1	0	1	0	15
128			min	-1104.43	3	.101	15	-.104	3	0	15	0	3	0	4
129		8	max	1420.941	1	.377	4	.346	1	0	1	0	1	0	15
130			min	-1104.357	3	.092	15	-.104	3	0	15	0	3	0	4
131		9	max	1421.037	1	.339	4	.346	1	0	1	0	1	0	15
132			min	-1104.285	3	.083	15	-.104	3	0	15	0	3	0	4
133		10	max	1421.133	1	.301	4	.346	1	0	1	0	1	0	15
134			min	-1104.213	3	.074	15	-.104	3	0	15	0	3	0	4
135		11	max	1421.23	1	.263	4	.346	1	0	1	0	1	0	15
136			min	-1104.141	3	.066	15	-.104	3	0	15	0	3	0	4
137		12	max	1421.326	1	.225	4	.346	1	0	1	0	1	0	15
138			min	-1104.068	3	.057	15	-.104	3	0	15	0	3	0	4
139		13	max	1421.422	1	.188	4	.346	1	0	1	0	1	0	15
140			min	-1103.996	3	.048	15	-.104	3	0	15	0	3	0	4
141		14	max	1421.519	1	.15	4	.346	1	0	1	0	1	0	15
142			min	-1103.924	3	.03	9	-.104	3	0	15	0	3	0	4
143		15	max	1421.615	1	.118	10	.346	1	0	1	0	1	0	15
144			min	-1103.852	3	.005	1	-.104	3	0	15	0	3	0	4
145		16	max	1421.712	1	.094	10	.346	1	0	1	0	1	0	15
146			min	-1103.779	3	-.025	1	-.104	3	0	15	0	3	0	4
147		17	max	1421.808	1	.069	10	.346	1	0	1	0	1	0	15
148			min	-1103.707	3	-.054	1	-.104	3	0	15	0	3	0	4
149		18	max	1421.904	1	.045	10	.346	1	0	1	0	1	0	15
150			min	-1103.635	3	-.084	1	-.104	3	0	15	0	3	0	4
151		19	max	1422.001	1	.02	10	.346	1	0	1	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152	M7	min	-1103.562	3	-.113	1	-.104	3	0	15	0	3	0	4
153		max	139.83	2	1.803	4	.015	1	0	2	0	2	0	4
154		min	-176.683	9	.426	15	-.007	3	0	3	0	3	0	15
155		max	139.763	2	1.625	4	.015	1	0	2	0	2	0	2
156		min	-176.739	9	.384	15	-.007	3	0	3	0	3	0	15
157		max	139.696	2	1.447	4	.015	1	0	2	0	2	0	10
158		min	-176.795	9	.342	15	-.007	3	0	3	0	3	0	9
159		max	139.629	2	1.269	4	.015	1	0	2	0	2	0	10
160		min	-176.851	9	.3	15	-.007	3	0	3	0	3	0	1
161		max	139.562	2	1.091	4	.015	1	0	2	0	2	0	15
162		min	-176.907	9	.258	15	-.007	3	0	3	0	3	0	1
163		max	139.495	2	.913	4	.015	1	0	2	0	2	0	15
164		min	-176.963	9	.217	15	-.007	3	0	3	0	3	0	4
165		max	139.428	2	.735	4	.015	1	0	2	0	2	0	15
166		min	-177.019	9	.175	15	-.007	3	0	3	0	3	0	4
167		max	139.361	2	.557	4	.015	1	0	2	0	2	0	15
168		min	-177.075	9	.133	15	-.007	3	0	3	0	3	0	4
169	M8	max	139.293	2	.379	4	.015	1	0	2	0	2	0	15
170		min	-177.131	9	.091	15	-.007	3	0	3	0	3	-.001	4
171		max	139.226	2	.201	4	.015	1	0	2	0	2	0	15
172		min	-177.186	9	.049	15	-.007	3	0	3	0	3	-.001	4
173		max	139.159	2	.048	10	.015	1	0	2	0	2	0	15
174		min	-177.242	9	-.023	9	-.007	3	0	3	0	3	-.001	4
175		max	139.092	2	-.034	15	.015	1	0	2	0	2	0	15
176		min	-177.298	9	-.161	1	-.007	3	0	3	0	3	-.001	4
177		max	139.025	2	-.076	15	.015	1	0	2	0	2	0	15
178		min	-177.354	9	-.333	4	-.007	3	0	3	0	3	-.001	4
179		max	138.958	2	-.118	15	.015	1	0	2	0	2	0	15
180		min	-177.41	9	-.511	4	-.007	3	0	3	0	3	-.001	4
181		max	138.891	2	-.16	15	.015	1	0	2	0	2	0	15
182		min	-177.466	9	-.689	4	-.007	3	0	3	0	3	0	4
183		max	138.824	2	-.202	15	.015	1	0	2	0	2	0	15
184		min	-177.522	9	-.867	4	-.007	3	0	3	0	3	0	4
185	M8	max	138.757	2	-.244	15	.015	1	0	2	0	2	0	15
186		min	-177.578	9	-1.045	4	-.007	3	0	3	0	3	0	4
187		max	138.69	2	-.286	15	.015	1	0	2	0	2	0	15
188		min	-177.634	9	-1.223	4	-.007	3	0	3	0	3	0	4
189		max	138.622	2	-.327	15	.015	1	0	2	0	2	0	1
190		min	-177.69	9	-1.401	4	-.007	3	0	3	0	3	0	1
191		max	1756.583	1	0	1	.717	1	0	1	0	15	0	1
192		min	-355.4	3	0	1	-.187	3	0	1	0	1	0	1
193		max	1756.647	1	0	1	.717	1	0	1	0	1	0	1
194		min	-355.352	3	0	1	-.187	3	0	1	0	3	0	1
195		max	1756.712	1	0	1	.717	1	0	1	0	1	0	1
196		min	-355.303	3	0	1	-.187	3	0	1	0	3	0	1
197		max	1756.777	1	0	1	.717	1	0	1	0	1	0	1
198		min	-355.255	3	0	1	-.187	3	0	1	0	3	0	1
199		max	1756.842	1	0	1	.717	1	0	1	0	1	0	1
200		min	-355.206	3	0	1	-.187	3	0	1	0	3	0	1
201	M8	max	1756.906	1	0	1	.717	1	0	1	0	1	0	1
202		min	-355.158	3	0	1	-.187	3	0	1	0	3	0	1
203		max	1756.971	1	0	1	.717	1	0	1	0	1	0	1
204		min	-355.109	3	0	1	-.187	3	0	1	0	3	0	1
205		max	1757.036	1	0	1	.717	1	0	1	0	1	0	1
206		min	-355.061	3	0	1	-.187	3	0	1	0	3	0	1
207		max	1757.1	1	0	1	.717	1	0	1	0	1	0	1
208		min	-355.012	3	0	1	-.187	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1757.165	1	0	1	.717	1	0	1	0	1	0	1
210			min	-354.964	3	0	1	-.187	3	0	1	0	3	0	1
211		11	max	1757.23	1	0	1	.717	1	0	1	0	1	0	1
212			min	-354.915	3	0	1	-.187	3	0	1	0	3	0	1
213		12	max	1757.295	1	0	1	.717	1	0	1	0	1	0	1
214			min	-354.867	3	0	1	-.187	3	0	1	0	3	0	1
215		13	max	1757.359	1	0	1	.717	1	0	1	0	1	0	1
216			min	-354.818	3	0	1	-.187	3	0	1	0	3	0	1
217		14	max	1757.424	1	0	1	.717	1	0	1	0	1	0	1
218			min	-354.77	3	0	1	-.187	3	0	1	0	3	0	1
219		15	max	1757.489	1	0	1	.717	1	0	1	0	1	0	1
220			min	-354.721	3	0	1	-.187	3	0	1	0	3	0	1
221		16	max	1757.553	1	0	1	.717	1	0	1	0	1	0	1
222			min	-354.673	3	0	1	-.187	3	0	1	0	3	0	1
223		17	max	1757.618	1	0	1	.717	1	0	1	.001	1	0	1
224			min	-354.624	3	0	1	-.187	3	0	1	0	3	0	1
225		18	max	1757.683	1	0	1	.717	1	0	1	.001	1	0	1
226			min	-354.575	3	0	1	-.187	3	0	1	0	3	0	1
227		19	max	1757.747	1	0	1	.717	1	0	1	.001	1	0	1
228			min	-354.527	3	0	1	-.187	3	0	1	0	3	0	1
229	M10	1	max	443.202	1	.647	4	-.004	15	.001	1	0	2	0	1
230			min	-327.761	3	.155	15	-.13	1	0	3	0	3	0	1
231		2	max	443.299	1	.609	4	-.004	15	.001	1	0	2	0	15
232			min	-327.688	3	.146	15	-.13	1	0	3	0	3	0	4
233		3	max	443.395	1	.571	4	-.004	15	.001	1	0	2	0	15
234			min	-327.616	3	.137	15	-.13	1	0	3	0	3	0	4
235		4	max	443.492	1	.533	4	-.004	15	.001	1	0	2	0	15
236			min	-327.544	3	.128	15	-.13	1	0	3	0	3	0	4
237		5	max	443.588	1	.495	4	-.004	15	.001	1	0	2	0	15
238			min	-327.471	3	.12	15	-.13	1	0	3	0	1	0	4
239		6	max	443.684	1	.458	4	-.004	15	.001	1	0	15	0	15
240			min	-327.399	3	.111	15	-.13	1	0	3	0	1	0	4
241		7	max	443.781	1	.42	4	-.004	15	.001	1	0	15	0	15
242			min	-327.327	3	.102	15	-.13	1	0	3	0	1	0	4
243		8	max	443.877	1	.382	4	-.004	15	.001	1	0	15	0	15
244			min	-327.255	3	.093	15	-.13	1	0	3	0	1	0	4
245		9	max	443.973	1	.344	4	-.004	15	.001	1	0	15	0	15
246			min	-327.182	3	.084	15	-.13	1	0	3	0	1	0	4
247		10	max	444.07	1	.306	4	-.004	15	.001	1	0	15	0	15
248			min	-327.11	3	.075	15	-.13	1	0	3	0	1	0	4
249		11	max	444.166	1	.268	4	-.004	15	.001	1	0	15	0	15
250			min	-327.038	3	.066	15	-.13	1	0	3	0	1	0	4
251		12	max	444.262	1	.231	4	-.004	15	.001	1	0	15	0	15
252			min	-326.966	3	.057	15	-.13	1	0	3	0	1	0	4
253		13	max	444.359	1	.193	4	-.004	15	.001	1	0	15	0	15
254			min	-326.893	3	.048	15	-.13	1	0	3	0	1	0	4
255		14	max	444.455	1	.155	4	-.004	15	.001	1	0	15	0	15
256			min	-326.821	3	.023	1	-.13	1	0	3	0	1	0	4
257		15	max	444.552	1	.117	4	-.004	15	.001	1	0	15	0	15
258			min	-326.749	3	-.007	1	-.13	1	0	3	0	1	0	4
259		16	max	444.648	1	.093	3	-.004	15	.001	1	0	15	0	15
260			min	-326.676	3	-.036	1	-.13	1	0	3	0	1	0	4
261		17	max	444.744	1	.071	3	-.004	15	.001	1	0	15	0	15
262			min	-326.604	3	-.066	1	-.13	1	0	3	0	1	0	4
263		18	max	444.841	1	.049	3	-.004	15	.001	1	0	15	0	15
264			min	-326.532	3	-.095	1	-.13	1	0	3	0	1	0	4
265		19	max	444.937	1	.027	3	-.004	15	.001	1	0	15	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-326.46	3	-.125	1	-.13	1	0	3	0	1	0	4
267		1	max	31.503	10	1.816	4	.932	1	.001	1	0	3	0	4
268			min	-132.425	1	.427	15	.023	12	0	15	-.002	1	0	15
269		2	max	31.447	10	1.638	4	.932	1	.001	1	0	3	0	4
270			min	-132.492	1	.386	15	.023	12	0	15	-.002	1	0	15
271		3	max	31.391	10	1.46	4	.932	1	.001	1	0	3	0	2
272			min	-132.559	1	.344	15	.023	12	0	15	-.002	1	0	12
273		4	max	31.335	10	1.282	4	.932	1	.001	1	0	3	0	15
274			min	-132.626	1	.302	15	.023	12	0	15	-.002	1	0	4
275		5	max	31.279	10	1.104	4	.932	1	.001	1	0	3	0	15
276			min	-132.693	1	.26	15	.023	12	0	15	-.001	1	0	4
277	6	max	31.223	10	.926	4	.932	1	.001	1	0	3	0	15	
278		min	-132.76	1	.218	15	.023	12	0	15	-.001	1	0	4	
279	7	max	31.167	10	.748	4	.932	1	.001	1	0	3	0	15	
280		min	-132.827	1	.176	15	.023	12	0	15	-.001	1	0	4	
281	8	max	31.111	10	.57	4	.932	1	.001	1	0	3	0	15	
282		min	-132.894	1	.135	15	.023	12	0	15	0	1	0	4	
283	9	max	31.055	10	.392	4	.932	1	.001	1	0	3	0	15	
284		min	-132.961	1	.093	15	.023	12	0	15	0	1	-.001	4	
285	10	max	30.999	10	.214	4	.932	1	.001	1	0	3	0	15	
286		min	-133.028	1	.051	15	.023	12	0	15	0	1	-.001	4	
287	11	max	30.943	10	.048	2	.932	1	.001	1	0	3	0	15	
288		min	-133.095	1	.004	12	.023	12	0	15	0	1	-.001	4	
289	12	max	30.887	10	-.033	15	.932	1	.001	1	0	3	0	15	
290		min	-133.163	1	-.142	4	.023	12	0	15	0	2	-.001	4	
291	13	max	30.832	10	-.075	15	.932	1	.001	1	0	1	0	15	
292		min	-133.23	1	-.32	4	.023	12	0	15	0	10	-.001	4	
293	14	max	30.776	10	-.117	15	.932	1	.001	1	0	1	0	15	
294		min	-133.297	1	-.498	4	.023	12	0	15	0	15	-.001	4	
295	15	max	30.72	10	-.158	15	.932	1	.001	1	0	1	0	15	
296		min	-133.364	1	-.676	4	.023	12	0	15	0	15	0	4	
297	16	max	30.664	10	-.2	15	.932	1	.001	1	0	1	0	15	
298		min	-133.431	1	-.854	4	.023	12	0	15	0	15	0	4	
299	17	max	30.608	10	-.242	15	.932	1	.001	1	0	1	0	15	
300		min	-133.498	1	-1.032	4	.023	12	0	15	0	15	0	4	
301	18	max	30.552	10	-.284	15	.932	1	.001	1	.001	1	0	15	
302		min	-133.565	1	-1.21	4	.023	12	0	15	0	15	0	4	
303	19	max	30.496	10	-.326	15	.932	1	.001	1	.001	1	0	1	
304		min	-133.632	1	-1.388	4	.023	12	0	15	0	15	0	1	
305	M12	1	max	589.454	1	0	1	3.719	1	0	1	0	1	0	1
306		min	-100.911	3	0	1	.113	15	0	1	0	3	0	1	
307	2	max	589.519	1	0	1	3.719	1	0	1	0	1	0	1	
308		min	-100.862	3	0	1	.113	15	0	1	0	12	0	1	
309	3	max	589.584	1	0	1	3.719	1	0	1	0	1	0	1	
310		min	-100.814	3	0	1	.113	15	0	1	0	15	0	1	
311	4	max	589.648	1	0	1	3.719	1	0	1	.001	1	0	1	
312		min	-100.765	3	0	1	.113	15	0	1	0	15	0	1	
313	5	max	589.713	1	0	1	3.719	1	0	1	.001	1	0	1	
314		min	-100.717	3	0	1	.113	15	0	1	0	15	0	1	
315	6	max	589.778	1	0	1	3.719	1	0	1	.002	1	0	1	
316		min	-100.668	3	0	1	.113	15	0	1	0	15	0	1	
317	7	max	589.842	1	0	1	3.719	1	0	1	.002	1	0	1	
318		min	-100.62	3	0	1	.113	15	0	1	0	15	0	1	
319	8	max	589.907	1	0	1	3.719	1	0	1	.002	1	0	1	
320		min	-100.571	3	0	1	.113	15	0	1	0	15	0	1	
321	9	max	589.972	1	0	1	3.719	1	0	1	.003	1	0	1	
322		min	-100.523	3	0	1	.113	15	0	1	0	15	0	1	







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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	134.742	1	5.582	9	68.402	1	0	1	.004	1	.199	1
438			min	-4.664	3	-22.707	3	1.14	12	0	15	0	10	-.111	3
439		11	max	134.814	1	5.357	9	68.402	1	0	1	.019	1	.199	1
440			min	-4.61	3	-22.909	3	1.14	12	0	15	0	15	-.106	3
441		12	max	134.886	1	5.133	9	68.402	1	0	1	.033	1	.199	1
442			min	-4.556	3	-23.111	3	1.14	12	0	15	.001	15	-.101	3
443		13	max	134.958	1	4.908	9	68.402	1	0	1	.048	1	.199	1
444			min	-4.501	3	-23.314	3	1.14	12	0	15	.001	15	-.096	3
445		14	max	135.031	1	4.683	9	68.402	1	0	1	.063	1	.199	1
446			min	-4.447	3	-23.516	3	1.14	12	0	15	.002	15	-.091	3
447		15	max	135.103	1	4.458	9	68.402	1	0	1	.078	1	.2	1
448			min	-4.393	3	-23.718	3	1.14	12	0	15	.002	15	-.086	3
449		16	max	63.796	2	7.566	10	69.279	1	0	15	.095	1	.201	1
450			min	-31.833	3	-89.967	1	1.165	12	0	1	.003	15	-.08	3
451		17	max	63.868	2	7.341	10	69.279	1	0	15	.11	1	.22	1
452			min	-31.779	3	-90.237	1	1.165	12	0	1	.003	15	-.07	3
453		18	max	-3.631	15	477.986	1	72.897	1	0	1	.125	1	.119	1
454			min	-117.833	1	-146.618	3	1.352	12	0	3	.004	15	-.038	3
455		19	max	-3.609	15	477.716	1	72.897	1	0	1	.141	1	.015	1
456			min	-117.761	1	-146.821	3	1.352	12	0	3	.004	15	-.006	3
457	M13	1	max	98.426	1	429.716	1	-3.615	15	.014	1	.144	1	0	1
458			min	3.115	15	-315.081	3	-117.922	1	-.009	3	.004	15	0	3
459		2	max	98.426	1	303.074	1	-2.772	15	.014	1	.046	1	.254	3
460			min	3.115	15	-222.163	3	-90.393	1	-.009	3	.001	15	-.346	1
461		3	max	98.426	1	176.433	1	-1.93	15	.014	1	0	3	.42	3
462			min	3.115	15	-129.245	3	-62.864	1	-.009	3	-.027	1	-.572	1
463		4	max	98.426	1	49.791	1	-1.087	15	.014	1	-.001	12	.498	3
464			min	3.115	15	-36.327	3	-35.335	1	-.009	3	-.073	1	-.679	1
465		5	max	98.426	1	56.591	3	-.245	15	.014	1	-.002	12	.488	3
466			min	3.115	15	-76.851	1	-7.805	1	-.009	3	-.094	1	-.666	1
467		6	max	98.426	1	149.509	3	19.724	1	.014	1	-.002	12	.391	3
468			min	3.115	15	-203.492	1	.316	12	-.009	3	-.088	1	-.534	1
469		7	max	98.426	1	242.427	3	47.253	1	.014	1	-.001	12	.206	3
470			min	3.115	15	-330.134	1	1.138	12	-.009	3	-.056	1	-.282	1
471		8	max	98.426	1	335.345	3	74.782	1	.014	1	.001	1	.089	1
472			min	3.115	15	-456.776	1	1.96	12	-.009	3	0	12	-.067	3
473		9	max	98.426	1	428.263	3	102.311	1	.014	1	.085	1	.581	1
474			min	3.115	15	-583.417	1	2.782	12	-.009	3	.002	12	-.428	3
475		10	max	98.426	1	521.181	3	129.84	1	.011	2	.195	1	1.191	1
476			min	3.115	15	-710.059	1	3.604	12	-.014	1	.005	12	-.876	3
477		11	max	73.361	1	583.417	1	-2.704	12	.009	3	.081	1	.581	1
478			min	2.263	15	-428.263	3	-101.762	1	-.014	1	0	12	-.428	3
479		12	max	73.361	1	456.776	1	-1.882	12	.009	3	0	10	.089	1
480			min	2.263	15	-335.345	3	-74.233	1	-.014	1	-.002	1	-.067	3
481		13	max	73.361	1	330.134	1	-1.06	12	.009	3	-.002	15	.206	3
482			min	2.263	15	-242.427	3	-46.704	1	-.014	1	-.059	1	-.282	1
483		14	max	73.361	1	203.492	1	-.238	12	.009	3	-.003	15	.391	3
484			min	2.263	15	-149.509	3	-19.175	1	-.014	1	-.09	1	-.534	1
485		15	max	73.361	1	76.851	1	8.354	1	.009	3	-.003	15	.488	3
486			min	2.263	15	-56.591	3	.263	15	-.014	1	-.095	1	-.666	1
487		16	max	73.361	1	36.327	3	35.884	1	.009	3	-.002	12	.498	3
488			min	2.263	15	-49.791	1	1.105	15	-.014	1	-.074	1	-.679	1
489		17	max	73.361	1	129.245	3	63.413	1	.009	3	0	12	.42	3
490			min	2.263	15	-176.433	1	1.948	15	-.014	1	-.028	1	-.572	1
491		18	max	73.361	1	222.163	3	90.942	1	.009	3	.045	1	.254	3
492			min	2.263	15	-303.074	1	2.79	15	-.014	1	.001	15	-.346	1
493		19	max	73.361	1	315.081	3	118.471	1	.009	3	.144	1	0	1



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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	1	min	2.263	15	-429.716	1	3.632	15	-.014	1	.004	15	0	3
495			max	-1.352	12	478.336	1	-3.609	15	.006	3	.141	1	0	1
496			min	-72.668	1	-146.835	3	-117.771	1	-.015	1	.004	15	0	3
497		2	max	-1.352	12	337.353	1	-2.767	15	.006	3	.043	1	.118	3
498			min	-72.668	1	-103.632	3	-90.242	1	-.015	1	.001	15	-.385	1
499		3	max	-1.352	12	196.37	1	-1.924	15	.006	3	0	12	.196	3
500			min	-72.668	1	-60.43	3	-62.713	1	-.015	1	-.029	1	-.637	1
501		4	max	-1.352	12	55.387	1	-1.082	15	.006	3	-.002	15	.232	3
502			min	-72.668	1	-17.227	3	-35.184	1	-.015	1	-.075	1	-.756	1
503		5	max	-1.352	12	25.975	3	-.239	15	.006	3	-.003	15	.228	3
504			min	-72.668	1	-85.596	1	-7.655	1	-.015	1	-.096	1	-.742	1
505		6	max	-1.352	12	69.177	3	19.874	1	.006	3	-.003	15	.183	3
506	min		-72.668	1	-226.578	1	.404	12	-.015	1	-.09	1	-.594	1	
507	7	max	-1.352	12	112.38	3	47.403	1	.006	3	-.002	15	.098	3	
508		min	-72.668	1	-367.561	1	1.226	12	-.015	1	-.058	1	-.314	1	
509	8	max	-1.352	12	155.582	3	74.932	1	.006	3	0	2	.1	1	
510		min	-72.668	1	-508.544	1	2.048	12	-.015	1	-.001	3	-.029	3	
511	9	max	-1.352	12	198.785	3	102.461	1	.006	3	.083	1	.647	1	
512		min	-72.668	1	-649.527	1	2.87	12	-.015	1	.002	12	-.196	3	
513	10	max	-2.314	15	-17.629	15	129.99	1	0	15	.193	1	1.327	1	
514		min	-74.957	1	-790.51	1	-5.666	3	-.015	1	.006	12	-.404	3	
515	11	max	-2.314	15	649.527	1	-2.961	12	.015	1	.084	1	.647	1	
516		min	-74.957	1	-198.785	3	-102.244	1	-.006	3	.002	12	-.196	3	
517	12	max	-2.314	15	508.544	1	-2.139	12	.015	1	0	2	.1	1	
518		min	-74.957	1	-155.582	3	-74.715	1	-.006	3	0	3	-.029	3	
519	13	max	-2.314	15	367.561	1	-1.317	12	.015	1	-.002	12	.098	3	
520		min	-74.957	1	-112.38	3	-47.186	1	-.006	3	-.057	1	-.314	1	
521	14	max	-2.314	15	226.578	1	-.495	12	.015	1	-.003	12	.183	3	
522		min	-74.957	1	-69.177	3	-19.657	1	-.006	3	-.089	1	-.594	1	
523	15	max	-2.314	15	85.595	1	7.872	1	.015	1	-.003	12	.228	3	
524		min	-74.957	1	-25.975	3	.246	15	-.006	3	-.094	1	-.742	1	
525	16	max	-2.314	15	17.227	3	35.401	1	.015	1	-.002	12	.232	3	
526		min	-74.957	1	-55.387	1	1.088	15	-.006	3	-.074	1	-.756	1	
527	17	max	-2.314	15	60.43	3	62.93	1	.015	1	0	12	.196	3	
528		min	-74.957	1	-196.37	1	1.931	15	-.006	3	-.028	1	-.637	1	
529	18	max	-2.314	15	103.632	3	90.459	1	.015	1	.045	1	.118	3	
530		min	-74.957	1	-337.353	1	2.773	15	-.006	3	.001	15	-.385	1	
531	19	max	-2.314	15	146.835	3	117.988	1	.015	1	.143	1	0	1	
532		min	-74.957	1	-478.336	1	3.615	12	-.006	3	.004	15	0	3	
533	M15	1	max	0	10	2.956	4	.019	3	0	1	0	1	0	1
534			min	-27.149	1	0	10	-.028	1	0	3	0	3	0	1
535	2	max	0	10	2.628	4	.019	3	0	1	0	1	0	10	
536		min	-27.221	1	0	10	-.028	1	0	3	0	3	-.001	4	
537	3	max	0	10	2.299	4	.019	3	0	1	0	1	0	10	
538		min	-27.293	1	0	10	-.028	1	0	3	0	3	-.003	4	
539	4	max	0	10	1.971	4	.019	3	0	1	0	1	0	10	
540		min	-27.365	1	0	10	-.028	1	0	3	0	3	-.004	4	
541	5	max	0	10	1.642	4	.019	3	0	1	0	1	0	10	
542		min	-27.437	1	0	10	-.028	1	0	3	0	3	-.005	4	
543	6	max	0	10	1.314	4	.019	3	0	1	0	1	0	10	
544		min	-27.509	1	0	10	-.028	1	0	3	0	3	-.005	4	
545	7	max	0	10	.985	4	.019	3	0	1	0	3	0	10	
546		min	-27.581	1	0	10	-.028	1	0	3	0	1	-.006	4	
547	8	max	0	10	.657	4	.019	3	0	1	0	3	0	10	
548		min	-27.653	1	0	10	-.028	1	0	3	0	1	-.006	4	
549	9	max	0	10	.328	4	.019	3	0	1	0	3	0	10	
550		min	-27.725	1	0	10	-.028	1	0	3	0	1	-.006	4	



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	10	0	1	.019	3	0	1	0	3	0	10
552			min	-27.797	1	0	1	-.028	1	0	3	0	1	-.007	4
553		11	max	0	10	0	10	.019	3	0	1	0	3	0	10
554			min	-27.869	1	-.328	4	-.028	1	0	3	0	1	-.006	4
555		12	max	0	10	0	10	.019	3	0	1	0	3	0	10
556			min	-27.941	1	-.657	4	-.028	1	0	3	0	1	-.006	4
557		13	max	0	10	0	10	.019	3	0	1	0	3	0	10
558			min	-28.013	1	-.985	4	-.028	1	0	3	0	1	-.006	4
559		14	max	0	10	0	10	.019	3	0	1	0	3	0	10
560			min	-28.085	1	-1.314	4	-.028	1	0	3	0	1	-.005	4
561		15	max	0	10	0	10	.019	3	0	1	0	3	0	10
562			min	-28.157	1	-1.642	4	-.028	1	0	3	0	1	-.005	4
563		16	max	0	10	0	10	.019	3	0	1	0	3	0	10
564			min	-28.229	1	-1.971	4	-.028	1	0	3	0	1	-.004	4
565		17	max	0	10	0	10	.019	3	0	1	0	3	0	10
566			min	-28.301	1	-2.299	4	-.028	1	0	3	0	1	-.003	4
567		18	max	0	10	0	10	.019	3	0	1	0	3	0	10
568			min	-28.373	1	-2.628	4	-.028	1	0	3	0	1	-.001	4
569		19	max	0	10	0	10	.019	3	0	1	0	3	0	1
570			min	-28.445	1	-2.956	4	-.028	1	0	3	0	1	0	1
571	M16A	1	max	-.797	10	2.956	4	.018	1	0	3	0	3	0	1
572			min	-31.913	1	.695	15	-.008	3	0	1	0	1	0	1
573		2	max	-.737	10	2.628	4	.018	1	0	3	0	3	0	15
574			min	-31.841	1	.618	15	-.008	3	0	1	0	1	-.001	4
575		3	max	-.677	10	2.299	4	.018	1	0	3	0	3	0	15
576			min	-31.769	1	.54	15	-.008	3	0	1	0	1	-.003	4
577		4	max	-.617	10	1.971	4	.018	1	0	3	0	3	0	15
578			min	-31.697	1	.463	15	-.008	3	0	1	0	1	-.004	4
579		5	max	-.557	10	1.642	4	.018	1	0	3	0	3	-.001	15
580			min	-31.625	1	.386	15	-.008	3	0	1	0	1	-.005	4
581		6	max	-.497	10	1.314	4	.018	1	0	3	0	3	-.001	15
582			min	-31.553	1	.309	15	-.008	3	0	1	0	1	-.005	4
583		7	max	-.437	10	.985	4	.018	1	0	3	0	3	-.001	15
584			min	-31.482	1	.232	15	-.008	3	0	1	0	1	-.006	4
585		8	max	-.377	10	.657	4	.018	1	0	3	0	3	-.001	15
586			min	-31.41	1	.154	15	-.008	3	0	1	0	1	-.006	4
587		9	max	-.317	10	.328	4	.018	1	0	3	0	3	-.002	15
588			min	-31.338	1	.077	15	-.008	3	0	1	0	1	-.006	4
589		10	max	-.257	10	0	1	.018	1	0	3	0	3	-.002	15
590			min	-31.266	1	0	1	-.008	3	0	1	0	1	-.007	4
591		11	max	-.198	10	-.077	15	.018	1	0	3	0	3	-.002	15
592			min	-31.194	1	-.328	4	-.008	3	0	1	0	1	-.006	4
593		12	max	-.138	10	-.154	15	.018	1	0	3	0	3	-.001	15
594			min	-31.122	1	-.657	4	-.008	3	0	1	0	1	-.006	4
595		13	max	-.078	10	-.232	15	.018	1	0	3	0	1	-.001	15
596			min	-31.05	1	-.985	4	-.008	3	0	1	0	13	-.006	4
597		14	max	-.018	10	-.309	15	.018	1	0	3	0	1	-.001	15
598			min	-30.978	1	-1.314	4	-.008	3	0	1	0	3	-.005	4
599		15	max	.042	10	-.386	15	.018	1	0	3	0	1	-.001	15
600			min	-30.906	1	-1.642	4	-.008	3	0	1	0	3	-.005	4
601		16	max	.102	10	-.463	15	.018	1	0	3	0	1	0	15
602			min	-30.834	1	-1.971	4	-.008	3	0	1	0	3	-.004	4
603		17	max	.162	10	-.54	15	.018	1	0	3	0	1	0	15
604			min	-30.762	1	-2.299	4	-.008	3	0	1	0	3	-.003	4
605		18	max	.222	10	-.618	15	.018	1	0	3	0	1	0	15
606			min	-30.69	1	-2.628	4	-.008	3	0	1	0	3	-.001	4
607		19	max	.282	10	-.695	15	.018	1	0	3	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-30.618	1	-2.956	4	-.008	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.014	1	-3.239e-5	15	NC	3	NC	3	
2			min	-.002	3	-.004	3	0	3	-1.053e-3	1	5192.393	2	2211.604	1	
3			2	max	.003	1	.005	2	.013	1	-3.11e-5	15	NC	3	NC	3
4				min	-.002	3	-.004	3	0	3	-1.012e-3	1	5627.422	2	2397.472	1
5			3	max	.003	1	.005	2	.012	1	-2.981e-5	15	NC	3	NC	3
6				min	-.002	3	-.004	3	0	3	-9.703e-4	1	6138.202	2	2616.132	1
7			4	max	.003	1	.004	2	.01	1	-2.852e-5	15	NC	3	NC	3
8				min	-.002	3	-.004	3	0	3	-9.288e-4	1	6742.008	2	2875.572	1
9			5	max	.002	1	.004	2	.009	1	-2.723e-5	15	NC	3	NC	3
10				min	-.002	3	-.004	3	0	3	-8.872e-4	1	7461.584	2	3186.453	1
11		6	max	.002	1	.004	2	.008	1	-2.595e-5	15	NC	1	NC	3	
12			min	-.002	3	-.004	3	0	3	-8.457e-4	1	8327.359	2	3563.26	1	
13		7	max	.002	1	.003	2	.007	1	-2.466e-5	15	NC	1	NC	2	
14			min	-.002	3	-.003	3	0	3	-8.042e-4	1	9380.818	2	4026.093	1	
15		8	max	.002	1	.003	2	.007	1	-2.337e-5	15	NC	1	NC	2	
16			min	-.002	3	-.003	3	0	3	-7.626e-4	1	NC	1	4603.514	1	
17		9	max	.002	1	.002	2	.006	1	-2.208e-5	15	NC	1	NC	2	
18			min	-.001	3	-.003	3	0	3	-7.211e-4	1	NC	1	5337.267	1	
19		10	max	.002	1	.002	2	.005	1	-2.079e-5	15	NC	1	NC	2	
20			min	-.001	3	-.003	3	0	3	-6.796e-4	1	NC	1	6290.41	1	
21		11	max	.001	1	.002	2	.004	1	-1.95e-5	15	NC	1	NC	2	
22			min	-.001	3	-.003	3	0	3	-6.38e-4	1	NC	1	7562.038	1	
23		12	max	.001	1	.001	2	.003	1	-1.822e-5	15	NC	1	NC	2	
24			min	0	3	-.002	3	0	3	-5.965e-4	1	NC	1	9315.594	1	
25		13	max	.001	1	.001	2	.003	1	-1.693e-5	15	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-5.55e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.002	1	-1.564e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-5.134e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	-1.435e-5	15	NC	1	NC	1	
30			min	0	3	-.001	3	0	3	-4.719e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.306e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-4.304e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.178e-5	15	NC	1	NC	1	
34			min	0	3	0	3	0	12	-3.888e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.043e-5	12	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.473e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-7.463e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.058e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.39e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.49e-6	12	NC	1	NC	1	
41			2	max	0	1	0	2	0	12	1.773e-4	1	NC	1	NC	1
42				min	0	10	0	3	0	1	5.27e-6	12	NC	1	NC	1
43			3	max	0	1	0	2	0	12	2.156e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	-.001	1	6.556e-6	15	NC	1	NC	1
45			4	max	0	1	0	2	0	12	2.538e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	-.001	1	7.742e-6	15	NC	1	NC	1
47			5	max	0	1	0	2	0	12	2.921e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	-.001	1	8.928e-6	15	NC	1	NC	1
49			6	max	0	1	0	2	0	3	3.304e-4	1	NC	1	NC	1
50				min	0	10	-.003	3	-.001	1	1.011e-5	15	NC	1	NC	1
51		7	max	0	1	0	2	0	3	3.686e-4	1	NC	1	NC	1	



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	10	-.004	3	0	1	1.13e-5	15	NC	1	NC	1
53	8	max	0	1	.001	2	0	3	4.069e-4	1	NC	1	NC	1
54		min	0	10	-.004	3	0	1	1.249e-5	15	NC	1	NC	1
55	9	max	0	1	.001	2	0	3	4.452e-4	1	NC	1	NC	1
56		min	0	10	-.005	3	0	2	1.367e-5	15	NC	1	NC	1
57	10	max	0	1	.002	2	0	1	4.835e-4	1	NC	1	NC	1
58		min	0	10	-.005	3	0	15	1.486e-5	15	NC	1	NC	1
59	11	max	0	1	.002	1	.001	1	5.217e-4	1	NC	1	NC	1
60		min	0	10	-.006	3	0	15	1.605e-5	15	NC	1	NC	1
61	12	max	0	1	.003	1	.002	1	5.6e-4	1	NC	1	NC	1
62		min	0	10	-.006	3	0	15	1.723e-5	15	NC	1	NC	1
63	13	max	.001	1	.004	1	.003	1	5.983e-4	1	NC	1	NC	1
64		min	0	10	-.006	3	0	15	1.842e-5	15	NC	1	NC	1
65	14	max	.001	1	.005	1	.003	1	6.365e-4	1	NC	1	NC	1
66		min	0	10	-.006	3	0	15	1.96e-5	15	NC	1	NC	1
67	15	max	.001	1	.005	1	.004	1	6.748e-4	1	NC	3	NC	1
68		min	0	10	-.006	3	0	15	2.079e-5	15	8410.33	1	NC	1
69	16	max	.001	1	.006	1	.005	1	7.131e-4	1	NC	3	NC	2
70		min	0	10	-.006	3	0	15	2.198e-5	15	7127.048	1	9094.333	1
71	17	max	.001	1	.008	1	.006	1	7.513e-4	1	NC	3	NC	2
72		min	0	10	-.007	3	0	15	2.316e-5	15	6135.245	1	7917.479	1
73	18	max	.001	1	.009	1	.007	1	7.896e-4	1	NC	3	NC	2
74		min	0	10	-.006	3	0	15	2.435e-5	15	5359.436	1	7058.555	1
75	19	max	.002	1	.01	1	.007	1	8.279e-4	1	NC	3	NC	2
76		min	0	10	-.006	3	0	15	2.553e-5	15	4746.834	1	6422.798	1
77	M4	1	max	.003	1	.007	2	15	-2.836e-5	15	NC	1	NC	3
78		min	0	3	-.005	3	-.005	1	-9.289e-4	1	NC	1	3600.878	1
79	2	max	.003	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
80		min	0	3	-.005	3	-.005	1	-9.289e-4	1	NC	1	3928.23	1
81	3	max	.002	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
82		min	0	3	-.004	3	-.004	1	-9.289e-4	1	NC	1	4317.833	1
83	4	max	.002	1	.006	2	0	15	-2.836e-5	15	NC	1	NC	2
84		min	0	3	-.004	3	-.004	1	-9.289e-4	1	NC	1	4786.103	1
85	5	max	.002	1	.005	2	0	15	-2.836e-5	15	NC	1	NC	2
86		min	0	3	-.004	3	-.004	1	-9.289e-4	1	NC	1	5355.411	1
87	6	max	.002	1	.005	2	0	15	-2.836e-5	15	NC	1	NC	2
88		min	0	3	-.003	3	-.003	1	-9.289e-4	1	NC	1	6056.867	1
89	7	max	.002	1	.005	2	0	15	-2.836e-5	15	NC	1	NC	2
90		min	0	3	-.003	3	-.003	1	-9.289e-4	1	NC	1	6934.788	1
91	8	max	.002	1	.004	2	0	15	-2.836e-5	15	NC	1	NC	2
92		min	0	3	-.003	3	-.002	1	-9.289e-4	1	NC	1	8054.074	1
93	9	max	.002	1	.004	2	0	15	-2.836e-5	15	NC	1	NC	2
94		min	0	3	-.003	3	-.002	1	-9.289e-4	1	NC	1	9512.896	1
95	10	max	.001	1	.003	2	0	15	-2.836e-5	15	NC	1	NC	1
96		min	0	3	-.002	3	-.002	1	-9.289e-4	1	NC	1	NC	1
97	11	max	.001	1	.003	2	0	15	-2.836e-5	15	NC	1	NC	1
98		min	0	3	-.002	3	-.001	1	-9.289e-4	1	NC	1	NC	1
99	12	max	.001	1	.003	2	0	15	-2.836e-5	15	NC	1	NC	1
100		min	0	3	-.002	3	-.001	1	-9.289e-4	1	NC	1	NC	1
101	13	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-9.289e-4	1	NC	1	NC	1
103	14	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
104		min	0	3	-.001	3	0	1	-9.289e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-2.836e-5	15	NC	1	NC	1
106		min	0	3	-.001	3	0	1	-9.289e-4	1	NC	1	NC	1
107	16	max	0	1	.001	2	0	15	-2.836e-5	15	NC	1	NC	1
108		min	0	3	0	3	0	1	-9.289e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-2.836e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-9.289e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-2.836e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-9.289e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.836e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.289e-4	1	NC	1	NC	1
115	M6	1	max	.011	1	.019	2	.004	1	2.334e-4	1	NC	3	NC	2
116			min	-.008	3	-.012	3	-.002	3	4.696e-6	10	1605.253	2	7669.618	1
117		2	max	.01	1	.018	2	.004	1	2.188e-4	1	NC	3	NC	2
118			min	-.008	3	-.012	3	-.002	3	3.883e-6	10	1712.19	2	8320.523	1
119		3	max	.009	1	.016	2	.003	1	2.042e-4	1	NC	3	NC	2
120			min	-.007	3	-.011	3	-.001	3	3.07e-6	10	1834.044	2	9094.021	1
121		4	max	.009	1	.015	2	.003	1	1.895e-4	1	NC	3	NC	1
122			min	-.007	3	-.011	3	-.001	3	2.258e-6	10	1973.789	2	NC	1
123		5	max	.008	1	.014	2	.003	1	1.749e-4	1	NC	3	NC	1
124			min	-.006	3	-.01	3	-.001	3	1.445e-6	10	2135.251	2	NC	1
125		6	max	.008	1	.013	2	.002	1	1.603e-4	1	NC	3	NC	1
126			min	-.006	3	-.009	3	-.001	3	6.32e-7	10	2323.439	2	NC	1
127		7	max	.007	1	.012	2	.002	1	1.457e-4	1	NC	3	NC	1
128			min	-.005	3	-.009	3	-.001	3	-1.807e-7	10	2545.032	2	NC	1
129		8	max	.006	1	.011	2	.002	1	1.311e-4	1	NC	3	NC	1
130			min	-.005	3	-.008	3	0	3	-9.935e-7	10	2809.141	2	NC	1
131		9	max	.006	1	.01	2	.002	1	1.165e-4	1	NC	3	NC	1
132			min	-.005	3	-.007	3	0	3	-1.806e-6	10	3128.525	2	NC	1
133		10	max	.005	1	.009	2	.001	1	1.047e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	0	3	-2.619e-6	10	3521.613	2	NC	1
135		11	max	.005	1	.007	2	.001	1	1.014e-4	3	NC	3	NC	1
136			min	-.004	3	-.006	3	0	3	-3.432e-6	10	4016.048	2	NC	1
137		12	max	.004	1	.006	2	0	1	9.818e-5	3	NC	3	NC	1
138			min	-.003	3	-.005	3	0	3	-6.017e-6	2	4655.267	2	NC	1
139		13	max	.004	1	.005	2	0	1	9.494e-5	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-1.072e-5	2	5511.66	2	NC	1
141		14	max	.003	1	.004	2	0	1	9.17e-5	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-1.542e-5	2	6715.522	2	NC	1
143		15	max	.002	1	.004	2	0	1	8.845e-5	3	NC	3	NC	1
144			min	-.002	3	-.003	3	0	3	-2.012e-5	2	8527.425	2	NC	1
145		16	max	.002	1	.003	2	0	1	8.521e-5	3	NC	1	NC	1
146			min	-.001	3	-.002	3	0	3	-2.482e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	1	0	1	8.197e-5	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-2.952e-5	2	NC	1	NC	1
149		18	max	0	1	0	1	0	1	7.873e-5	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-3.423e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	7.549e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.893e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.742e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-3.419e-5	3	NC	1	NC	1
155		2	max	0	9	.001	1	0	3	1.481e-5	2	NC	1	NC	1
156			min	0	2	-.001	3	0	2	-2.609e-5	3	NC	1	NC	1
157		3	max	0	9	.003	1	0	3	1.367e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-1.799e-5	3	NC	1	NC	1
159		4	max	0	9	.004	1	0	3	1.406e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	2	-9.887e-6	3	NC	1	NC	1
161		5	max	0	9	.005	1	0	3	1.444e-5	1	NC	3	NC	1
162			min	0	2	-.005	3	0	2	-1.786e-6	3	9086.935	1	NC	1
163		6	max	0	9	.006	1	0	3	1.482e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	3.806e-7	15	7189.832	1	NC	1
165		7	max	0	9	.008	1	0	3	1.52e-5	1	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	4.299e-7	15	5898.823	1	NC	1
167		8	max	0	9	.009	1	0	3	2.252e-5	3	NC	3	NC	1
168			min	0	2	-.009	3	0	1	-8.689e-7	2	4959.07	1	NC	1
169		9	max	0	9	.011	1	0	3	3.062e-5	3	NC	3	NC	1
170			min	0	2	-.01	3	0	1	-3.482e-6	2	4243.065	1	NC	1
171		10	max	.001	9	.013	1	.001	3	3.872e-5	3	NC	3	NC	1
172			min	0	2	-.011	3	0	1	-6.095e-6	2	3679.698	1	NC	1
173		11	max	.001	9	.014	1	.001	3	4.682e-5	3	NC	3	NC	1
174			min	0	2	-.012	3	-.001	1	-8.708e-6	2	3225.952	1	NC	1
175		12	max	.001	9	.016	1	.001	3	5.493e-5	3	NC	3	NC	1
176			min	0	2	-.013	3	-.001	1	-1.132e-5	2	2854.09	1	NC	1
177		13	max	.001	9	.018	1	.001	3	6.303e-5	3	NC	3	NC	1
178			min	-.001	2	-.014	3	-.001	1	-1.393e-5	2	2545.286	1	NC	1
179		14	max	.001	9	.02	1	.001	3	7.113e-5	3	NC	3	NC	1
180			min	-.001	2	-.015	3	-.001	1	-1.655e-5	2	2286.214	1	NC	1
181		15	max	.002	9	.022	1	.001	3	7.923e-5	3	NC	3	NC	1
182			min	-.001	2	-.016	3	-.002	1	-1.916e-5	2	2067.123	1	NC	1
183		16	max	.002	9	.024	1	.001	3	8.733e-5	3	NC	3	NC	1
184			min	-.001	2	-.016	3	-.002	1	-2.177e-5	2	1880.674	1	NC	1
185		17	max	.002	9	.027	1	.001	3	9.543e-5	3	NC	3	NC	1
186			min	-.001	2	-.017	3	-.002	1	-2.439e-5	2	1721.228	1	NC	1
187		18	max	.002	9	.029	1	0	3	1.035e-4	3	NC	3	NC	1
188			min	-.001	2	-.018	3	-.002	1	-2.7e-5	2	1584.372	1	NC	1
189		19	max	.002	9	.031	1	0	3	1.116e-4	3	NC	3	NC	1
190			min	-.002	2	-.018	3	-.002	1	-2.961e-5	2	1466.614	1	NC	1
191	M8	1	max	.008	1	.022	1	.002	1	-3.048e-7	10	NC	1	NC	2
192			min	-.002	3	-.014	3	0	3	-1.103e-4	1	NC	1	8542.405	1
193		2	max	.008	1	.021	1	.002	1	-3.048e-7	10	NC	1	NC	2
194			min	-.002	3	-.013	3	0	3	-1.103e-4	1	NC	1	9313.557	1
195		3	max	.007	1	.02	1	.002	1	-3.048e-7	10	NC	1	NC	1
196			min	-.002	3	-.012	3	0	3	-1.103e-4	1	NC	1	NC	1
197		4	max	.007	1	.019	1	.002	1	-3.048e-7	10	NC	1	NC	1
198			min	-.001	3	-.011	3	0	3	-1.103e-4	1	NC	1	NC	1
199		5	max	.007	1	.017	1	.002	1	-3.048e-7	10	NC	1	NC	1
200			min	-.001	3	-.011	3	0	3	-1.103e-4	1	NC	1	NC	1
201		6	max	.006	1	.016	1	.001	1	-3.048e-7	10	NC	1	NC	1
202			min	-.001	3	-.01	3	0	3	-1.103e-4	1	NC	1	NC	1
203		7	max	.006	1	.015	1	.001	1	-3.048e-7	10	NC	1	NC	1
204			min	-.001	3	-.009	3	0	3	-1.103e-4	1	NC	1	NC	1
205		8	max	.005	1	.014	1	.001	1	-3.048e-7	10	NC	1	NC	1
206			min	-.001	3	-.008	3	0	3	-1.103e-4	1	NC	1	NC	1
207		9	max	.005	1	.012	1	0	1	-3.048e-7	10	NC	1	NC	1
208			min	0	3	-.008	3	0	3	-1.103e-4	1	NC	1	NC	1
209		10	max	.004	1	.011	1	0	1	-3.048e-7	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.103e-4	1	NC	1	NC	1
211		11	max	.004	1	.01	1	0	1	-3.048e-7	10	NC	1	NC	1
212			min	0	3	-.006	3	0	3	-1.103e-4	1	NC	1	NC	1
213		12	max	.003	1	.009	1	0	1	-3.048e-7	10	NC	1	NC	1
214			min	0	3	-.005	3	0	3	-1.103e-4	1	NC	1	NC	1
215		13	max	.003	1	.007	1	0	1	-3.048e-7	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.103e-4	1	NC	1	NC	1
217		14	max	.002	1	.006	1	0	1	-3.048e-7	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.103e-4	1	NC	1	NC	1
219		15	max	.002	1	.005	1	0	1	-3.048e-7	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.103e-4	1	NC	1	NC	1
221		16	max	.001	1	.004	1	0	1	-3.048e-7	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.103e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.002	1	0	1	-3.048e-7	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.103e-4	1	NC	1	NC	1
225		18	max	0	1	.001	1	0	1	-3.048e-7	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.103e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-3.048e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.103e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.006	2	0	3	9.256e-4	1	NC	3	NC	1
230			min	-.002	3	-.004	3	-.002	1	-1.33e-4	3	5202.877	2	NC	1
231		2	max	.003	1	.005	2	0	3	8.774e-4	1	NC	3	NC	1
232			min	-.002	3	-.004	3	-.002	1	-1.297e-4	3	5623.109	2	NC	1
233		3	max	.003	1	.005	2	0	3	8.292e-4	1	NC	3	NC	1
234			min	-.002	3	-.004	3	-.002	1	-1.264e-4	3	6113.758	2	NC	1
235		4	max	.003	1	.004	2	0	3	7.81e-4	1	NC	3	NC	1
236			min	-.002	3	-.004	3	-.001	1	-1.232e-4	3	6690.238	2	NC	1
237		5	max	.003	1	.004	2	0	3	7.327e-4	1	NC	3	NC	1
238			min	-.002	3	-.004	3	-.001	1	-1.199e-4	3	7372.678	2	NC	1
239		6	max	.002	1	.004	2	0	3	6.845e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	-.001	1	-1.166e-4	3	8187.781	2	NC	1
241		7	max	.002	1	.003	2	0	3	6.363e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	-.001	1	-1.133e-4	3	9171.626	2	NC	1
243		8	max	.002	1	.003	2	0	3	5.881e-4	1	NC	1	NC	1
244			min	-.001	3	-.003	3	0	1	-1.101e-4	3	NC	1	NC	1
245		9	max	.002	1	.003	2	0	3	5.399e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-1.068e-4	3	NC	1	NC	1
247		10	max	.002	1	.002	2	0	3	4.917e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-1.035e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.435e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-1.002e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	3.952e-4	1	NC	1	NC	1
252			min	0	3	-.002	3	0	1	-9.697e-5	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	3.47e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-9.37e-5	3	NC	1	NC	1
255		14	max	0	1	0	2	0	3	2.988e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-9.043e-5	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.506e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-8.716e-5	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.024e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-8.388e-5	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.542e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-8.061e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.059e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-7.734e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	5.773e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-7.407e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	3.375e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.775e-5	1	NC	1	NC	1
269		2	max	0	1	0	2	0	2	2.371e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-9.481e-5	1	NC	1	NC	1
271		3	max	0	1	0	2	0	2	1.367e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-1.619e-4	1	NC	1	NC	1
273		4	max	0	1	0	2	0	10	3.634e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	1	-2.289e-4	1	NC	1	NC	1
275		5	max	0	1	0	2	0	10	-4.568e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	-.001	1	-2.96e-4	1	NC	1	NC	1
277		6	max	0	1	0	2	0	15	-1.097e-5	12	NC	1	NC	1
278			min	0	10	-.003	3	-.002	1	-3.63e-4	1	NC	1	NC	1
279		7	max	0	1	0	2	0	15	-1.321e-5	15	NC	1	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-0.004	3	-0.003	1	-4.301e-4	1	NC	1	NC	1
281		8	max	0	1	.001	2	0	15	-1.53e-5	15	NC	1	NC	1
282			min	0	10	-0.005	3	-0.003	1	-4.971e-4	1	NC	1	NC	1
283		9	max	0	1	.002	1	0	15	-1.739e-5	15	NC	1	NC	1
284			min	0	10	-0.005	3	-0.004	1	-5.642e-4	1	NC	1	NC	1
285		10	max	0	1	.002	1	0	15	-1.948e-5	15	NC	1	NC	2
286			min	0	10	-0.005	3	-0.005	1	-6.313e-4	1	NC	1	8464.661	1
287		11	max	0	1	.003	1	0	15	-2.157e-5	15	NC	1	NC	2
288			min	0	10	-0.006	3	-0.007	1	-6.983e-4	1	NC	1	7053.649	1
289		12	max	0	1	.003	1	0	15	-2.366e-5	15	NC	1	NC	2
290			min	0	10	-0.006	3	-0.008	1	-7.654e-4	1	NC	1	6027.713	1
291		13	max	.001	1	.004	1	0	15	-2.575e-5	15	NC	1	NC	2
292			min	0	10	-0.006	3	-0.009	1	-8.324e-4	1	NC	1	5260.66	1
293		14	max	.001	1	.005	1	0	15	-2.784e-5	15	NC	2	NC	2
294			min	0	10	-0.006	3	-.01	1	-8.995e-4	1	9796.775	1	4675.023	1
295		15	max	.001	1	.006	1	0	15	-2.992e-5	15	NC	3	NC	2
296			min	0	10	-0.007	3	-0.011	1	-9.665e-4	1	8240.03	1	4221.182	1
297		16	max	.001	1	.007	1	0	15	-3.201e-5	15	NC	3	NC	2
298			min	0	10	-0.007	3	-.012	1	-1.034e-3	1	7034.229	1	3866.287	1
299		17	max	.001	1	.008	1	0	15	-3.41e-5	15	NC	3	NC	2
300			min	0	10	-0.007	3	-.013	1	-1.101e-3	1	6090.108	1	3588.059	1
301		18	max	.001	1	.009	1	0	15	-3.619e-5	15	NC	3	NC	3
302			min	0	10	-0.007	3	-.014	1	-1.168e-3	1	5343.982	1	3371.187	1
303		19	max	.002	1	.01	1	0	15	-3.828e-5	15	NC	3	NC	3
304			min	0	10	-0.006	3	-.014	1	-1.235e-3	1	4750.022	1	3205.166	1
305	M12	1	max	.003	1	.007	2	.012	1	1.057e-3	1	NC	1	NC	3
306			min	0	3	-0.005	3	0	15	3.317e-5	15	NC	1	1630.353	1
307		2	max	.003	1	.006	2	.011	1	1.057e-3	1	NC	1	NC	3
308			min	0	3	-0.005	3	0	15	3.317e-5	15	NC	1	1778.014	1
309		3	max	.002	1	.006	2	.01	1	1.057e-3	1	NC	1	NC	3
310			min	0	3	-0.004	3	0	15	3.317e-5	15	NC	1	1953.781	1
311		4	max	.002	1	.006	2	.009	1	1.057e-3	1	NC	1	NC	3
312			min	0	3	-0.004	3	0	15	3.317e-5	15	NC	1	2165.062	1
313		5	max	.002	1	.005	2	.008	1	1.057e-3	1	NC	1	NC	3
314			min	0	3	-0.004	3	0	15	3.317e-5	15	NC	1	2421.948	1
315		6	max	.002	1	.005	2	.007	1	1.057e-3	1	NC	1	NC	3
316			min	0	3	-0.003	3	0	15	3.317e-5	15	NC	1	2738.479	1
317		7	max	.002	1	.005	2	.006	1	1.057e-3	1	NC	1	NC	3
318			min	0	3	-0.003	3	0	15	3.317e-5	15	NC	1	3134.647	1
319		8	max	.002	1	.004	2	.005	1	1.057e-3	1	NC	1	NC	3
320			min	0	3	-0.003	3	0	15	3.317e-5	15	NC	1	3639.733	1
321		9	max	.002	1	.004	2	.004	1	1.057e-3	1	NC	1	NC	2
322			min	0	3	-0.003	3	0	15	3.317e-5	15	NC	1	4298.03	1
323		10	max	.001	1	.003	2	.004	1	1.057e-3	1	NC	1	NC	2
324			min	0	3	-0.002	3	0	15	3.317e-5	15	NC	1	5179.169	1
325		11	max	.001	1	.003	2	.003	1	1.057e-3	1	NC	1	NC	2
326			min	0	3	-0.002	3	0	15	3.317e-5	15	NC	1	6397.848	1
327		12	max	.001	1	.003	2	.002	1	1.057e-3	1	NC	1	NC	2
328			min	0	3	-0.002	3	0	15	3.317e-5	15	NC	1	8154.16	1
329		13	max	0	1	.002	2	.002	1	1.057e-3	1	NC	1	NC	1
330			min	0	3	-0.002	3	0	15	3.317e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	.001	1	1.057e-3	1	NC	1	NC	1
332			min	0	3	-0.001	3	0	15	3.317e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	1.057e-3	1	NC	1	NC	1
334			min	0	3	-0.001	3	0	15	3.317e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	1.057e-3	1	NC	1	NC	1
336			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	1.057e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.057e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.317e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.057e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.317e-5	15	NC	1	NC	1
343	M1	1	max	.005	3	.02	3	0	3	2.137e-2	1	NC	1	NC	1
344			min	-.006	2	-.031	1	-.004	1	-1.559e-2	3	NC	1	NC	1
345		2	max	.004	3	.011	3	0	3	1.028e-2	1	NC	4	NC	2
346			min	-.006	2	-.017	1	-.01	1	-7.715e-3	3	3138.913	1	8285.438	1
347		3	max	.004	3	.002	3	0	3	1.29e-5	3	NC	5	NC	2
348			min	-.006	2	-.003	1	-.014	1	-5.914e-4	1	1623.801	1	5022.214	1
349		4	max	.004	3	.009	1	0	3	1.586e-5	3	NC	5	NC	2
350			min	-.006	2	-.005	3	-.016	1	-4.855e-4	1	1149.439	1	4153.195	1
351		5	max	.005	3	.019	1	0	12	1.882e-5	3	NC	5	NC	2
352			min	-.006	2	-.011	3	-.016	1	-3.795e-4	1	921.6	1	3984.362	1
353		6	max	.005	3	.027	1	0	12	2.179e-5	3	NC	5	NC	2
354			min	-.006	2	-.016	3	-.015	1	-2.736e-4	1	792.972	1	4257.344	1
355		7	max	.005	3	.033	1	0	12	2.475e-5	3	NC	5	NC	2
356			min	-.006	2	-.019	3	-.013	1	-1.676e-4	1	715.252	1	5057.371	1
357		8	max	.005	3	.038	1	0	3	2.772e-5	3	NC	5	NC	2
358			min	-.006	2	-.022	3	-.011	1	-6.167e-5	1	668.367	1	6910.825	1
359		9	max	.005	3	.04	1	0	3	4.428e-5	1	NC	5	NC	1
360			min	-.006	2	-.023	3	-.008	1	1.587e-6	15	642.955	1	NC	1
361		10	max	.005	3	.041	1	0	3	1.502e-4	1	NC	5	NC	1
362			min	-.006	2	-.023	3	-.004	1	4.832e-6	15	634.711	1	NC	1
363		11	max	.005	3	.04	1	0	3	2.562e-4	1	NC	5	NC	1
364			min	-.006	2	-.022	3	-.001	1	8.078e-6	15	642.352	1	NC	1
365		12	max	.005	3	.037	1	.002	1	3.621e-4	1	NC	5	NC	2
366			min	-.006	2	-.02	3	0	15	1.132e-5	15	667.079	1	8173.273	1
367		13	max	.005	3	.033	1	.005	1	4.681e-4	1	NC	5	NC	2
368			min	-.006	2	-.018	3	0	15	1.457e-5	15	713.076	1	5653.15	1
369		14	max	.005	3	.026	1	.006	1	5.74e-4	1	NC	5	NC	2
370			min	-.006	2	-.014	3	0	15	1.781e-5	15	789.489	1	4626.071	1
371		15	max	.005	3	.018	1	.007	1	6.8e-4	1	NC	5	NC	2
372			min	-.006	2	-.01	3	0	15	2.106e-5	15	915.903	1	4255.672	1
373		16	max	.005	3	.008	1	.007	1	7.58e-4	1	NC	5	NC	2
374			min	-.006	2	-.004	3	0	15	2.346e-5	15	1138.975	1	4381.937	1
375		17	max	.005	3	.002	3	.005	1	1.718e-4	1	NC	5	NC	2
376			min	-.006	2	-.004	1	0	15	5.888e-6	15	1598.628	1	5253.429	1
377		18	max	.005	3	.008	3	.002	1	1.185e-2	1	NC	4	NC	2
378			min	-.006	2	-.018	1	0	15	-3.649e-3	3	3081.485	1	8615.431	1
379		19	max	.005	3	.015	3	0	3	2.377e-2	1	NC	1	NC	1
380			min	-.006	2	-.034	1	-.003	1	-7.393e-3	3	NC	1	NC	1
381	M5	1	max	.013	3	.061	3	0	3	4.22e-7	1	NC	1	NC	1
382			min	-.019	2	-.095	1	-.005	1	3.587e-8	15	NC	1	NC	1
383		2	max	.013	3	.033	3	.001	3	3.323e-5	3	NC	5	NC	1
384			min	-.019	2	-.051	1	-.005	1	-9.602e-5	1	1055.273	1	NC	1
385		3	max	.013	3	.007	3	.002	3	6.549e-5	3	NC	5	NC	1
386			min	-.019	2	-.009	1	-.004	1	-1.904e-4	1	543.548	1	NC	1
387		4	max	.013	3	.026	1	.002	3	6.51e-5	3	NC	5	NC	1
388			min	-.019	2	-.014	3	-.003	1	-1.774e-4	1	383.961	1	NC	1
389		5	max	.013	3	.056	1	.002	3	6.472e-5	3	NC	15	NC	1
390			min	-.019	2	-.031	3	-.003	1	-1.644e-4	1	307.293	1	NC	1
391		6	max	.013	3	.081	1	.002	3	6.434e-5	3	NC	15	NC	1
392			min	-.019	2	-.045	3	-.003	1	-1.513e-4	1	263.957	1	NC	1
393		7	max	.013	3	.1	1	.002	3	6.395e-5	3	NC	15	NC	1







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	1	-.013	15	.016	1	7.622e-3	3	7993.466	15	NC	4
566		min	0	10	-.058	4	-.01	3	-1.256e-2	1	1878.984	4	3136.185	1
567	18	max	0	1	-.007	15	.002	9	8.082e-3	3	NC	5	NC	4
568		min	0	10	-.03	4	-.004	2	-1.334e-2	1	3692.493	4	5579.435	1
569	19	max	0	1	.004	3	.014	3	8.542e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.02	1	-1.412e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.005	3	2.882e-3	3	NC	1	NC	1
572		min	0	1	-.002	1	-.006	2	-4.431e-3	1	NC	1	NC	1
573	2	max	0	10	-.007	15	.006	1	2.749e-3	3	NC	5	NC	2
574		min	0	1	-.029	4	0	10	-4.212e-3	1	3692.493	4	9447.225	1
575	3	max	0	10	-.013	15	.015	1	2.617e-3	3	7993.466	15	NC	3
576		min	0	1	-.057	4	-.004	3	-3.993e-3	1	1878.984	4	5341.426	1
577	4	max	0	10	-.019	15	.022	1	2.485e-3	3	5483.984	15	NC	4
578		min	0	1	-.083	4	-.007	3	-3.774e-3	1	1289.092	4	4059.257	1
579	5	max	0	10	-.025	15	.026	1	2.353e-3	3	4279.209	15	NC	4
580		min	0	1	-.106	4	-.009	3	-3.555e-3	1	1005.892	4	3502.49	1
581	6	max	0	10	-.03	15	.029	1	2.221e-3	3	3601.406	15	NC	4
582		min	0	1	-.126	4	-.01	3	-3.337e-3	1	846.564	4	3257.816	1
583	7	max	0	10	-.033	15	.03	1	2.089e-3	3	3193.798	15	NC	4
584		min	0	1	-.142	4	-.011	3	-3.118e-3	1	750.75	4	3195.545	1
585	8	max	0	10	-.036	15	.029	1	1.957e-3	3	2949.171	15	NC	4
586		min	0	1	-.154	4	-.011	3	-2.899e-3	1	693.247	4	3271.051	1
587	9	max	0	10	-.038	15	.028	1	1.825e-3	3	2817.499	15	NC	4
588		min	0	1	-.161	4	-.01	3	-2.68e-3	1	662.295	4	3477.782	1
589	10	max	0	10	-.038	15	.025	1	1.693e-3	3	2775.844	15	NC	4
590		min	0	1	-.163	4	-.009	3	-2.461e-3	1	652.504	4	3836.203	1
591	11	max	0	10	-.038	15	.022	1	1.561e-3	3	2817.499	15	NC	3
592		min	0	1	-.161	4	-.008	3	-2.242e-3	1	662.295	4	4398.063	1
593	12	max	0	10	-.036	15	.018	1	1.429e-3	3	2949.171	15	NC	3
594		min	0	1	-.154	4	-.007	3	-2.023e-3	1	693.247	4	5266.176	1
595	13	max	0	10	-.033	15	.014	1	1.297e-3	3	3193.798	15	NC	2
596		min	0	1	-.142	4	-.005	3	-1.804e-3	1	750.75	4	6645.16	1
597	14	max	0	10	-.03	15	.01	1	1.164e-3	3	3601.406	15	NC	2
598		min	0	1	-.126	4	-.004	3	-1.585e-3	1	846.564	4	8975.679	1
599	15	max	0	10	-.025	15	.007	1	1.032e-3	3	4279.209	15	NC	1
600		min	0	1	-.106	4	-.002	3	-1.366e-3	1	1005.892	4	NC	1
601	16	max	0	10	-.019	15	.004	1	9.003e-4	3	5483.984	15	NC	1
602		min	0	1	-.083	4	-.001	3	-1.147e-3	1	1289.092	4	NC	1
603	17	max	0	10	-.013	15	.001	1	7.682e-4	3	7993.466	15	NC	1
604		min	0	1	-.057	4	0	3	-9.284e-4	1	1878.984	4	NC	1
605	18	max	0	10	-.007	15	0	4	6.361e-4	3	NC	5	NC	1
606		min	0	1	-.029	4	0	2	-7.095e-4	1	3692.493	4	NC	1
607	19	max	0	1	0	1	0	1	5.04e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.905e-4	1	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
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Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

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