



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	120 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.793 k-ft
$M_z$ =	0.402 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>99%</b>

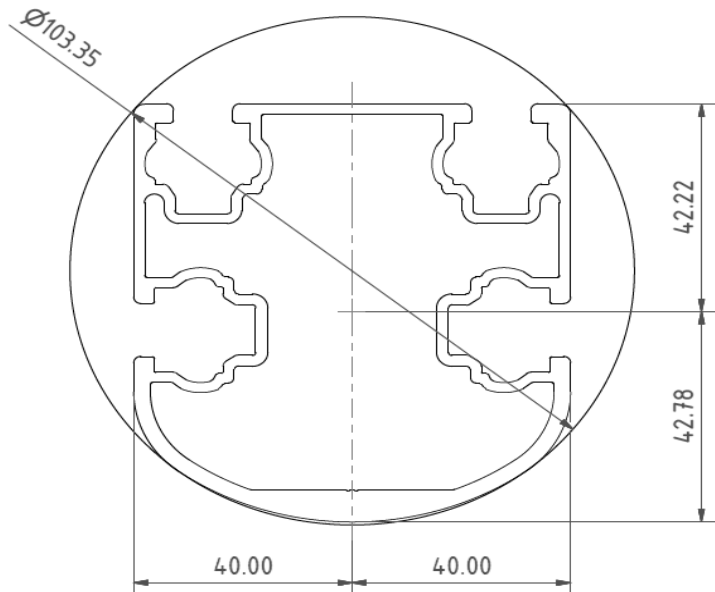


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.259 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.870 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>97%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.868 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>10%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.342 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>40%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.209 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<b>37%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

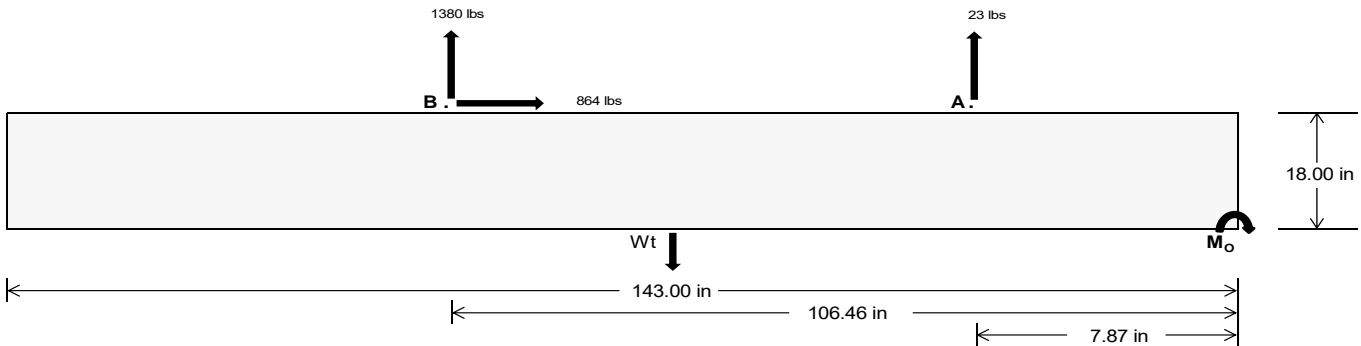
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>109.50</b>	<b>5754.93</b>	k
Compressive Load =	<b>3728.58</b>	<b>4775.15</b>	k
Lateral Load =	<b>24.35</b>	<b>3595.29</b>	k
Moment (Weak Axis) =	<b>0.05</b>	<b>0.01</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 162671.4$  in-lbs  
Resisting Force Required = 2275.12 lbs  
S.F. = 1.67  
Weight Required = 3791.87 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Sliding

Force = 864.02 lbs  
Friction = 0.4  
Weight Required = 2160.04 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 864.02 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$  7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1398 lbs	1398 lbs	1398 lbs	1398 lbs	1230 lbs	1230 lbs	1230 lbs	1230 lbs	1825 lbs	1825 lbs	1825 lbs	1825 lbs	-46 lbs	-46 lbs	-46 lbs	-46 lbs
$F_B$	1394 lbs	1394 lbs	1394 lbs	1394 lbs	2022 lbs	2022 lbs	2022 lbs	2022 lbs	2418 lbs	2418 lbs	2418 lbs	2418 lbs	-2761 lbs	-2761 lbs	-2761 lbs	-2761 lbs
$F_V$	207 lbs	207 lbs	207 lbs	207 lbs	1577 lbs	1577 lbs	1577 lbs	1577 lbs	1318 lbs	1318 lbs	1318 lbs	1318 lbs	-1728 lbs	-1728 lbs	-1728 lbs	-1728 lbs
$P_{total}$	10352 lbs	10568 lbs	10784 lbs	11000 lbs	10812 lbs	11028 lbs	11244 lbs	11460 lbs	11802 lbs	12018 lbs	12234 lbs	12450 lbs	1730 lbs	1859 lbs	1989 lbs	2118 lbs
$M$	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	3000 lbs-ft	3000 lbs-ft	3000 lbs-ft	3000 lbs-ft	4612 lbs-ft	4612 lbs-ft	4612 lbs-ft	4612 lbs-ft	5208 lbs-ft	5208 lbs-ft	5208 lbs-ft	5208 lbs-ft
$e$	0.35 ft	0.35 ft	0.34 ft	0.33 ft	0.28 ft	0.27 ft	0.27 ft	0.26 ft	0.39 ft	0.38 ft	0.38 ft	0.37 ft	3.01 ft	2.80 ft	2.62 ft	2.46 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	244.8 psf	244.0 psf	243.3 psf	242.6 psf	267.6 psf	266.2 psf	264.9 psf	263.7 psf	272.8 psf	271.2 psf	269.8 psf	268.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	350.9 psf	347.2 psf	343.7 psf	340.4 psf	354.5 psf	350.7 psf	347.1 psf	343.7 psf	406.4 psf	401.1 psf	396.2 psf	391.5 psf	134.1 psf	130.9 psf	128.8 psf	127.4 psf

Maximum Bearing Pressure = 406 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

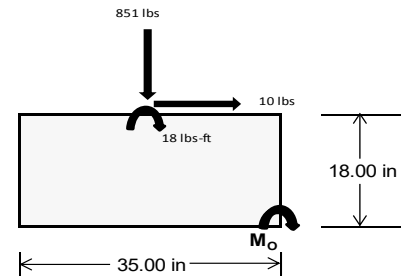
#### Overturning Check

$M_o = 1208.3 \text{ ft-lbs}$   
 Resisting Force Required = 828.53 lbs  
 S.F. = 1.67  
 Weight Required = 1380.88 lbs  
 Minimum Width = **35 in**  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	288 lbs	720 lbs	288 lbs	851 lbs	2344 lbs	851 lbs	84 lbs	211 lbs	84 lbs
$F_v$	3 lbs	0 lbs	3 lbs	10 lbs	0 lbs	10 lbs	1 lbs	0 lbs	1 lbs
$P_{total}$	9647 lbs	7560 lbs	9647 lbs	9760 lbs	7560 lbs	9760 lbs	2821 lbs	7560 lbs	2821 lbs
$M$	10 lbs-ft	0 lbs-ft	10 lbs-ft	33 lbs-ft	0 lbs-ft	33 lbs-ft	3 lbs-ft	0 lbs-ft	3 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	276.9 psf	217.5 psf	276.9 psf	278.9 psf	217.5 psf	278.9 psf	81.0 psf	217.5 psf	81.0 psf
$f_{max}$	278.2 psf	217.5 psf	278.2 psf	282.8 psf	217.5 psf	282.8 psf	81.4 psf	217.5 psf	81.4 psf



Maximum Bearing Pressure = 283 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 143in long x 29in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.580 k
Allowable Uplift =	1.214 k
Utilization =	<u>48%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.058 k
Allowable Uplift =	4.357 k
Utilization =	<u>47%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.868 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

#### Rear Strut

Maximum Axial Load =	3.866 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>52%</u>

#### Diagonal Strut

Maximum Axial Load =	2.436 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>33%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	60.93 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.219 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 120 \text{ in}$$

$$J = 0.432$$

$$331.976$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 120$$

$$J = 0.432$$

$$211.117$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 8.88 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 9.14 \text{ kips} \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.866	-46.866	0	0
2	M14	Y	-46.866	-46.866	0	0
3	M15	Y	-46.866	-46.866	0	0
4	M16	Y	-46.866	-46.866	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-47.984	-47.984	0	0
2	M14	y	-47.984	-47.984	0	0
3	M15	y	-77.191	-77.191	0	0
4	M16	y	-77.191	-77.191	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	108.485	108.485	0	0
2	M14	y	83.45	83.45	0	0
3	M15	y	45.897	45.897	0	0
4	M16	y	45.897	45.897	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes	Y		1	1.2	3	1.6	4	.8										
2	LRFD 1.2D + 1.6W + 0.5S	Yes	Y		1	1.2	3	.5	4	1.6										
3	LRFD 0.9D + 1.6W	Yes	Y		2	.9					5	1.6								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								



RISA-3D Version 13.0.0 [T:\...\PVMax 72 Cell 2V 30° 90mph 30psf 10ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	108.087	1	214.584	1	-.771	12	.015	2	-.009	15	.91	3
28			min	5.05	15	-303.352	3	-34.606	1	0	3	-.188	1	-.584	1
29		15	max	108.087	1	86.674	1	14.618	1	.015	2	-.009	12	1.143	3
30			min	5.05	15	-116.515	3	.701	15	0	3	-.199	1	-.752	1
31		16	max	108.087	1	70.321	3	63.841	1	.015	2	-.006	12	1.169	3
32			min	5.05	15	-41.236	1	2.983	15	0	3	-.155	1	-.777	1
33		17	max	108.087	1	257.157	3	113.065	1	.015	2	0	12	.987	3
34			min	5.05	15	-169.146	1	5.265	15	0	3	-.057	1	-.66	1
35		18	max	108.087	1	443.994	3	162.288	1	.015	2	.096	1	.597	3
36			min	5.05	15	-297.055	1	7.547	15	0	3	.005	15	-.401	1
37		19	max	108.087	1	630.83	3	211.511	1	.015	2	.304	1	0	1
38			min	5.05	15	-424.965	1	9.83	15	0	3	.014	15	0	3
39	M14	1	max	58.598	1	462.578	1	-10.181	15	.01	3	.354	1	0	1
40			min	2.746	15	-502.553	3	-219.078	1	-.013	2	.017	15	0	3
41		2	max	58.598	1	334.668	1	-7.899	15	.01	3	.138	1	.479	3
42			min	2.746	15	-360.224	3	-169.855	1	-.013	2	.006	15	-.443	1
43		3	max	58.598	1	206.758	1	-5.616	15	.01	3	.002	3	.8	3
44			min	2.746	15	-217.894	3	-120.631	1	-.013	2	-.024	1	-.744	1
45		4	max	58.598	1	78.848	1	-3.334	15	.01	3	-.005	12	.964	3
46			min	2.746	15	-75.565	3	-71.408	1	-.013	2	-.13	1	-.902	1
47		5	max	58.598	1	66.765	3	-1.052	15	.01	3	-.008	12	.968	3
48			min	2.746	15	-49.062	1	-22.184	1	-.013	2	-.182	1	-.919	1
49		6	max	58.598	1	209.095	3	27.039	1	.01	3	-.008	15	.815	3
50			min	2.746	15	-176.972	1	.42	12	-.013	2	-.179	1	-.793	1
51		7	max	58.598	1	351.424	3	76.262	1	.01	3	-.006	15	.504	3
52			min	2.746	15	-304.881	1	2.739	12	-.013	2	-.122	1	-.526	1
53		8	max	58.598	1	493.754	3	125.486	1	.01	3	0	10	.034	3
54			min	2.746	15	-432.791	1	5.058	12	-.013	2	-.01	1	-.128	2
55		9	max	58.598	1	636.084	3	174.709	1	.01	3	.157	1	.436	1
56			min	2.746	15	-560.701	1	7.378	12	-.013	2	.004	12	-.593	3
57		10	max	58.598	1	688.611	1	-9.697	12	.01	3	.378	1	1.13	1
58			min	2.746	15	-778.413	3	-223.933	1	-.013	2	.014	12	-1.379	3
59		11	max	58.598	1	560.701	1	-7.378	12	.013	2	.157	1	.436	1
60			min	2.746	15	-636.084	3	-174.709	1	-.01	3	.004	12	-.593	3
61		12	max	58.598	1	432.791	1	-5.058	12	.013	2	0	10	.034	3
62			min	2.746	15	-493.754	3	-125.486	1	-.01	3	-.01	1	-.128	2
63		13	max	58.598	1	304.881	1	-2.739	12	.013	2	-.006	15	.504	3
64			min	2.746	15	-351.424	3	-76.262	1	-.01	3	-.122	1	-.526	1
65		14	max	58.598	1	176.972	1	-.42	12	.013	2	-.008	15	.815	3
66			min	2.746	15	-209.095	3	-27.039	1	-.01	3	-.179	1	-.793	1
67		15	max	58.598	1	49.062	1	22.184	1	.013	2	-.008	12	.968	3
68			min	2.746	15	-66.765	3	1.052	15	-.01	3	-.182	1	-.919	1
69		16	max	58.598	1	75.565	3	71.408	1	.013	2	-.005	12	.964	3
70			min	2.746	15	-78.848	1	3.334	15	-.01	3	-.13	1	-.902	1
71		17	max	58.598	1	217.894	3	120.631	1	.013	2	.002	3	.8	3
72			min	2.746	15	-206.758	1	5.616	15	-.01	3	-.024	1	-.744	1
73		18	max	58.598	1	360.224	3	169.855	1	.013	2	.138	1	.479	3
74			min	2.746	15	-334.668	1	7.899	15	-.01	3	.006	15	-.443	1
75		19	max	58.598	1	502.553	3	219.078	1	.013	2	.354	1	0	1
76			min	2.746	15	-462.578	1	10.181	15	-.01	3	.017	15	0	3
77	M15	1	max	-2.947	15	615.085	2	-10.176	15	.014	2	.353	1	0	2
78			min	-62.859	1	-277.614	3	-219.003	1	-.009	3	.016	15	0	3
79		2	max	-2.947	15	442.206	2	-7.894	15	.014	2	.137	1	.266	3
80			min	-62.859	1	-202.045	3	-169.78	1	-.009	3	.006	15	-.587	2
81		3	max	-2.947	15	269.327	2	-5.611	15	.014	2	.002	3	.449	3
82			min	-62.859	1	-126.476	3	-120.556	1	-.009	3	-.024	1	-.983	2
83		4	max	-2.947	15	96.447	2	-3.329	15	.014	2	-.005	12	.548	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-62.859	1	-50.908	3	-71.333	1	-.009	3	-.131	1	-1.186	2
85		5	max	-2.947	15	24.661	3	-1.047	15	.014	2	-.008	12	.562	3
86			min	-62.859	1	-76.432	2	-22.11	1	-.009	3	-.182	1	-1.197	2
87		6	max	-2.947	15	100.23	3	27.114	1	.014	2	-.008	15	.493	3
88			min	-62.859	1	-249.312	2	.495	12	-.009	3	-.18	1	-1.016	2
89		7	max	-2.947	15	175.799	3	76.337	1	.014	2	-.006	15	.339	3
90			min	-62.859	1	-422.191	2	2.815	12	-.009	3	-.122	1	-.643	2
91		8	max	-2.947	15	251.367	3	125.561	1	.014	2	0	10	.102	3
92			min	-62.859	1	-595.07	2	5.134	12	-.009	3	-.01	1	-.09	1
93		9	max	-2.947	15	326.936	3	174.784	1	.014	2	.157	1	.679	2
94			min	-62.859	1	-767.95	2	7.453	12	-.009	3	.004	12	-.219	3
95		10	max	-2.947	15	940.829	2	-9.773	12	.014	2	.378	1	1.629	2
96			min	-62.859	1	-402.505	3	-224.007	1	-.009	3	.014	12	-.624	3
97		11	max	-2.947	15	767.95	2	-7.453	12	.009	3	.157	1	.679	2
98			min	-62.859	1	-326.936	3	-174.784	1	-.014	2	.004	12	-.219	3
99		12	max	-2.947	15	595.07	2	-5.134	12	.009	3	0	10	.102	3
100			min	-62.859	1	-251.367	3	-125.561	1	-.014	2	-.01	1	-.09	1
101		13	max	-2.947	15	422.191	2	-2.815	12	.009	3	-.006	15	.339	3
102			min	-62.859	1	-175.799	3	-76.337	1	-.014	2	-.122	1	-.643	2
103		14	max	-2.947	15	249.312	2	-.495	12	.009	3	-.008	15	.493	3
104			min	-62.859	1	-100.23	3	-27.114	1	-.014	2	-.18	1	-1.016	2
105		15	max	-2.947	15	76.432	2	22.11	1	.009	3	-.008	12	.562	3
106			min	-62.859	1	-24.661	3	1.047	15	-.014	2	-.182	1	-1.197	2
107		16	max	-2.947	15	50.908	3	71.333	1	.009	3	-.005	12	.548	3
108			min	-62.859	1	-96.447	2	3.329	15	-.014	2	-.131	1	-1.186	2
109		17	max	-2.947	15	126.476	3	120.556	1	.009	3	.002	3	.449	3
110			min	-62.859	1	-269.327	2	5.611	15	-.014	2	-.024	1	-.983	2
111		18	max	-2.947	15	202.045	3	169.78	1	.009	3	.137	1	.266	3
112			min	-62.859	1	-442.206	2	7.894	15	-.014	2	.006	15	-.587	2
113		19	max	-2.947	15	277.614	3	219.003	1	.009	3	.353	1	0	2
114			min	-62.859	1	-615.085	2	10.176	15	-.014	2	.016	15	0	3
115	M16	1	max	-5.698	15	577.868	2	-9.847	15	.012	1	.307	1	0	2
116			min	-121.647	1	-248.921	3	-.212	1	-.012	3	.014	15	0	3
117		2	max	-5.698	15	404.989	2	-7.565	15	.012	1	.098	1	.235	3
118			min	-121.647	1	-173.352	3	-162.777	1	-.012	3	.005	15	-.546	2
119		3	max	-5.698	15	232.11	2	-5.283	15	.012	1	-.001	12	.385	3
120			min	-121.647	1	-97.784	3	-113.554	1	-.012	3	-.055	1	-.9	2
121		4	max	-5.698	15	59.23	2	-3.001	15	.012	1	-.006	12	.452	3
122			min	-121.647	1	-22.215	3	-64.33	1	-.012	3	-.154	1	-1.062	2
123		5	max	-5.698	15	53.354	3	-.719	15	.012	1	-.009	12	.435	3
124			min	-121.647	1	-113.649	2	-15.107	1	-.012	3	-.198	1	-1.032	2
125		6	max	-5.698	15	128.923	3	34.117	1	.012	1	-.009	15	.333	3
126			min	-121.647	1	-286.529	2	.997	12	-.012	3	-.188	1	-.809	2
127		7	max	-5.698	15	204.491	3	83.34	1	.012	1	-.006	15	.148	3
128			min	-121.647	1	-459.408	2	3.316	12	-.012	3	-.122	1	-.395	2
129		8	max	-5.698	15	280.06	3	132.564	1	.012	1	.001	10	.212	2
130			min	-121.647	1	-632.288	2	5.635	12	-.012	3	-.003	3	-.121	3
131		9	max	-5.698	15	355.629	3	181.787	1	.012	1	.172	1	1.01	2
132			min	-121.647	1	-805.167	2	7.955	12	-.012	3	.006	12	-.474	3
133		10	max	-5.698	15	978.046	2	-10.274	12	.012	1	.402	1	2.001	2
134			min	-121.647	1	-431.198	3	-231.01	1	-.012	3	.016	12	-.911	3
135		11	max	-5.698	15	805.167	2	-7.955	12	.012	3	.172	1	1.01	2
136			min	-121.647	1	-355.629	3	-181.787	1	-.012	1	.006	12	-.474	3
137		12	max	-5.698	15	632.288	2	-5.635	12	.012	3	.001	10	.212	2
138			min	-121.647	1	-280.06	3	-132.564	1	-.012	1	-.003	3	-.121	3
139		13	max	-5.698	15	459.408	2	-3.316	12	.012	3	-.006	15	.148	3
140			min	-121.647	1	-204.491	3	-83.34	1	-.012	1	-.122	1	-.395	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-5.698	15	286.529	2	-.997	12	.012	3	-.009	15	.333	3
142			min	-121.647	1	-128.923	3	-34.117	1	-.012	1	-.188	1	-.809	2
143		15	max	-5.698	15	113.649	2	15.107	1	.012	3	-.009	12	.435	3
144			min	-121.647	1	-53.354	3	.719	15	-.012	1	-.198	1	-1.032	2
145		16	max	-5.698	15	22.215	3	64.33	1	.012	3	-.006	12	.452	3
146			min	-121.647	1	-59.23	2	3.001	15	-.012	1	-.154	1	-1.062	2
147		17	max	-5.698	15	97.784	3	113.554	1	.012	3	-.001	12	.385	3
148			min	-121.647	1	-232.11	2	5.283	15	-.012	1	-.055	1	-.9	2
149		18	max	-5.698	15	173.352	3	162.777	1	.012	3	.098	1	.235	3
150			min	-121.647	1	-404.989	2	7.565	15	-.012	1	.005	15	-.546	2
151		19	max	-5.698	15	248.921	3	212	1	.012	3	.307	1	0	2
152			min	-121.647	1	-577.868	2	9.847	15	-.012	1	.014	15	0	3
153	M2	1	max	1010.382	1	2.022	4	.605	1	0	5	0	3	0	1
154			min	-1195.944	3	.476	15	.028	15	0	1	0	1	0	1
155		2	max	1010.911	1	1.951	4	.605	1	0	5	0	1	0	15
156			min	-1195.547	3	.459	15	.028	15	0	1	0	15	0	4
157		3	max	1011.441	1	1.88	4	.605	1	0	5	0	1	0	15
158			min	-1195.15	3	.442	15	.028	15	0	1	0	15	-.001	4
159		4	max	1011.97	1	1.809	4	.605	1	0	5	0	1	0	15
160			min	-1194.753	3	.426	15	.028	15	0	1	0	15	-.002	4
161		5	max	1012.499	1	1.738	4	.605	1	0	5	0	1	0	15
162			min	-1194.356	3	.409	15	.028	15	0	1	0	15	-.003	4
163		6	max	1013.028	1	1.667	4	.605	1	0	5	.001	1	0	15
164			min	-1193.959	3	.392	15	.028	15	0	1	0	15	-.003	4
165		7	max	1013.558	1	1.596	4	.605	1	0	5	.001	1	0	15
166			min	-1193.562	3	.375	15	.028	15	0	1	0	15	-.004	4
167		8	max	1014.087	1	1.525	4	.605	1	0	5	.002	1	-.001	15
168			min	-1193.166	3	.359	15	.028	15	0	1	0	15	-.004	4
169		9	max	1014.616	1	1.454	4	.605	1	0	5	.002	1	-.001	15
170			min	-1192.769	3	.342	15	.028	15	0	1	0	15	-.005	4
171		10	max	1015.146	1	1.383	4	.605	1	0	5	.002	1	-.001	15
172			min	-1192.372	3	.325	15	.028	15	0	1	0	15	-.005	4
173		11	max	1015.675	1	1.312	4	.605	1	0	5	.002	1	-.001	15
174			min	-1191.975	3	.309	15	.028	15	0	1	0	15	-.006	4
175		12	max	1016.204	1	1.241	4	.605	1	0	5	.002	1	-.002	15
176			min	-1191.578	3	.292	15	.028	15	0	1	0	15	-.006	4
177		13	max	1016.733	1	1.17	4	.605	1	0	5	.003	1	-.002	15
178			min	-1191.181	3	.275	15	.028	15	0	1	0	15	-.007	4
179		14	max	1017.263	1	1.099	4	.605	1	0	5	.003	1	-.002	15
180			min	-1190.784	3	.259	15	.028	15	0	1	0	15	-.007	4
181		15	max	1017.792	1	1.028	4	.605	1	0	5	.003	1	-.002	15
182			min	-1190.387	3	.242	15	.028	15	0	1	0	15	-.008	4
183		16	max	1018.321	1	.957	4	.605	1	0	5	.003	1	-.002	15
184			min	-1189.99	3	.219	12	.028	15	0	1	0	15	-.008	4
185		17	max	1018.851	1	.886	4	.605	1	0	5	.003	1	-.002	15
186			min	-1189.593	3	.191	12	.028	15	0	1	0	15	-.008	4
187		18	max	1019.38	1	.815	4	.605	1	0	5	.004	1	-.002	15
188			min	-1189.196	3	.163	12	.028	15	0	1	0	15	-.009	4
189		19	max	1019.909	1	.758	2	.605	1	0	5	.004	1	-.002	15
190			min	-1188.799	3	.136	12	.028	15	0	1	0	15	-.009	4
191	M3	1	max	624.614	2	8.874	4	.472	1	0	15	0	1	.009	4
192			min	-785.835	3	2.086	15	.022	15	0	1	0	15	.002	15
193		2	max	624.444	2	8.005	4	.472	1	0	15	0	1	.005	4
194			min	-785.962	3	1.882	15	.022	15	0	1	0	15	0	12
195		3	max	624.273	2	7.136	4	.472	1	0	15	0	1	.002	2
196			min	-786.09	3	1.678	15	.022	15	0	1	0	15	0	3
197		4	max	624.103	2	6.267	4	.472	1	0	15	.001	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-786.218	3	1.473	15	.022	15	0	1	0	15	-.002	3
199		5	max	623.933	2	5.398	4	.472	1	0	15	.001	1	-.001	15
200			min	-786.346	3	1.269	15	.022	15	0	1	0	15	-.005	4
201		6	max	623.762	2	4.53	4	.472	1	0	15	.002	1	-.002	15
202			min	-786.473	3	1.065	15	.022	15	0	1	0	15	-.007	4
203		7	max	623.592	2	3.661	4	.472	1	0	15	.002	1	-.002	15
204			min	-786.601	3	.861	15	.022	15	0	1	0	15	-.009	4
205		8	max	623.422	2	2.792	4	.472	1	0	15	.002	1	-.002	15
206			min	-786.729	3	.656	15	.022	15	0	1	0	15	-.01	4
207		9	max	623.251	2	1.923	4	.472	1	0	15	.002	1	-.003	15
208			min	-786.857	3	.452	15	.022	15	0	1	0	15	-.011	4
209		10	max	623.081	2	1.054	4	.472	1	0	15	.002	1	-.003	15
210			min	-786.984	3	.248	15	.022	15	0	1	0	15	-.012	4
211		11	max	622.911	2	.281	2	.472	1	0	15	.003	1	-.003	15
212			min	-787.112	3	-.08	3	.022	15	0	1	0	15	-.012	4
213		12	max	622.74	2	-.16	15	.472	1	0	15	.003	1	-.003	15
214			min	-787.24	3	-.684	4	.022	15	0	1	0	15	-.012	4
215		13	max	622.57	2	-.365	15	.472	1	0	15	.003	1	-.003	15
216			min	-787.368	3	-1.553	4	.022	15	0	1	0	15	-.012	4
217		14	max	622.4	2	-.569	15	.472	1	0	15	.003	1	-.003	15
218			min	-787.495	3	-2.422	4	.022	15	0	1	0	15	-.011	4
219		15	max	622.229	2	-.773	15	.472	1	0	15	.004	1	-.002	15
220			min	-787.623	3	-3.29	4	.022	15	0	1	0	15	-.009	4
221		16	max	622.059	2	-.977	15	.472	1	0	15	.004	1	-.002	15
222			min	-787.751	3	-4.159	4	.022	15	0	1	0	15	-.008	4
223		17	max	621.888	2	-1.182	15	.472	1	0	15	.004	1	-.001	15
224			min	-787.879	3	-5.028	4	.022	15	0	1	0	15	-.006	4
225		18	max	621.718	2	-1.386	15	.472	1	0	15	.004	1	0	15
226			min	-788.006	3	-5.897	4	.022	15	0	1	0	15	-.003	4
227		19	max	621.548	2	-1.59	15	.472	1	0	15	.004	1	0	1
228			min	-788.134	3	-6.766	4	.022	15	0	1	0	15	0	1
229	M4	1	max	1120.407	1	0	1	-899	15	0	1	.004	1	0	1
230			min	3.982	3	0	1	-19.321	1	0	1	0	15	0	1
231		2	max	1120.577	1	0	1	-899	15	0	1	.001	1	0	1
232			min	4.11	3	0	1	-19.321	1	0	1	0	15	0	1
233		3	max	1120.748	1	0	1	-899	15	0	1	0	15	0	1
234			min	4.238	3	0	1	-19.321	1	0	1	0	1	0	1
235		4	max	1120.918	1	0	1	-899	15	0	1	0	15	0	1
236			min	4.366	3	0	1	-19.321	1	0	1	-.003	1	0	1
237		5	max	1121.088	1	0	1	-899	15	0	1	0	15	0	1
238			min	4.493	3	0	1	-19.321	1	0	1	-.005	1	0	1
239		6	max	1121.259	1	0	1	-899	15	0	1	0	15	0	1
240			min	4.621	3	0	1	-19.321	1	0	1	-.008	1	0	1
241		7	max	1121.429	1	0	1	-899	15	0	1	0	15	0	1
242			min	4.749	3	0	1	-19.321	1	0	1	-.01	1	0	1
243		8	max	1121.599	1	0	1	-899	15	0	1	0	15	0	1
244			min	4.877	3	0	1	-19.321	1	0	1	-.012	1	0	1
245		9	max	1121.77	1	0	1	-899	15	0	1	0	15	0	1
246			min	5.004	3	0	1	-19.321	1	0	1	-.014	1	0	1
247		10	max	1121.94	1	0	1	-899	15	0	1	0	15	0	1
248			min	5.132	3	0	1	-19.321	1	0	1	-.016	1	0	1
249		11	max	1122.11	1	0	1	-899	15	0	1	0	15	0	1
250			min	5.26	3	0	1	-19.321	1	0	1	-.019	1	0	1
251		12	max	1122.281	1	0	1	-899	15	0	1	0	15	0	1
252			min	5.388	3	0	1	-19.321	1	0	1	-.021	1	0	1
253		13	max	1122.451	1	0	1	-899	15	0	1	-.001	15	0	1
254			min	5.516	3	0	1	-19.321	1	0	1	-.023	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1122.621	1	0	1	-899	15	0	1	-.001	15	0	1
256		min	5.643	3	0	1	-19.321	1	0	1	-.025	1	0	1
257	15	max	1122.792	1	0	1	-899	15	0	1	-.001	15	0	1
258		min	5.771	3	0	1	-19.321	1	0	1	-.028	1	0	1
259	16	max	1122.962	1	0	1	-899	15	0	1	-.001	15	0	1
260		min	5.899	3	0	1	-19.321	1	0	1	-.03	1	0	1
261	17	max	1123.132	1	0	1	-899	15	0	1	-.001	15	0	1
262		min	6.027	3	0	1	-19.321	1	0	1	-.032	1	0	1
263	18	max	1123.303	1	0	1	-899	15	0	1	-.002	15	0	1
264		min	6.154	3	0	1	-19.321	1	0	1	-.034	1	0	1
265	19	max	1123.473	1	0	1	-899	15	0	1	-.002	15	0	1
266		min	6.282	3	0	1	-19.321	1	0	1	-.036	1	0	1
267	M6	1	max	3199.622	1	2.211	2	0	1	0	0	1	0	1
268		min	-3865.904	3	.301	12	0	1	0	1	0	1	0	1
269	2	max	3200.151	1	2.156	2	0	1	0	1	0	1	0	12
270		min	-3865.507	3	.273	12	0	1	0	1	0	1	0	2
271	3	max	3200.68	1	2.1	2	0	1	0	1	0	1	0	12
272		min	-3865.11	3	.246	12	0	1	0	1	0	1	-.002	2
273	4	max	3201.21	1	2.045	2	0	1	0	1	0	1	0	12
274		min	-3864.713	3	.218	12	0	1	0	1	0	1	-.002	2
275	5	max	3201.739	1	1.99	2	0	1	0	1	0	1	0	12
276		min	-3864.316	3	.19	12	0	1	0	1	0	1	-.003	2
277	6	max	3202.268	1	1.934	2	0	1	0	1	0	1	0	12
278		min	-3863.919	3	.163	12	0	1	0	1	0	1	-.004	2
279	7	max	3202.797	1	1.879	2	0	1	0	1	0	1	0	12
280		min	-3863.523	3	.135	12	0	1	0	1	0	1	-.004	2
281	8	max	3203.327	1	1.824	2	0	1	0	1	0	1	0	12
282		min	-3863.126	3	.107	12	0	1	0	1	0	1	-.005	2
283	9	max	3203.856	1	1.768	2	0	1	0	1	0	1	0	12
284		min	-3862.729	3	.07	3	0	1	0	1	0	1	-.006	2
285	10	max	3204.385	1	1.713	2	0	1	0	1	0	1	0	12
286		min	-3862.332	3	.029	3	0	1	0	1	0	1	-.006	2
287	11	max	3204.915	1	1.658	2	0	1	0	1	0	1	0	12
288		min	-3861.935	3	-.013	3	0	1	0	1	0	1	-.007	2
289	12	max	3205.444	1	1.602	2	0	1	0	1	0	1	0	12
290		min	-3861.538	3	-.054	3	0	1	0	1	0	1	-.008	2
291	13	max	3205.973	1	1.547	2	0	1	0	1	0	1	0	12
292		min	-3861.141	3	-.096	3	0	1	0	1	0	1	-.008	2
293	14	max	3206.502	1	1.492	2	0	1	0	1	0	1	0	12
294		min	-3860.744	3	-.137	3	0	1	0	1	0	1	-.009	2
295	15	max	3207.032	1	1.436	2	0	1	0	1	0	1	0	12
296		min	-3860.347	3	-.179	3	0	1	0	1	0	1	-.009	2
297	16	max	3207.561	1	1.381	2	0	1	0	1	0	1	0	3
298		min	-3859.95	3	-.22	3	0	1	0	1	0	1	-.01	2
299	17	max	3208.09	1	1.326	2	0	1	0	1	0	1	0	3
300		min	-3859.553	3	-.262	3	0	1	0	1	0	1	-.01	2
301	18	max	3208.62	1	1.27	2	0	1	0	1	0	1	0	3
302		min	-3859.156	3	-.303	3	0	1	0	1	0	1	-.011	2
303	19	max	3209.149	1	1.215	2	0	1	0	1	0	1	0	3
304		min	-3858.759	3	-.345	3	0	1	0	1	0	1	-.011	2
305	M7	1	max	2341.805	2	8.909	4	0	1	0	1	0	.011	2
306		min	-2434.069	3	2.091	15	0	1	0	1	0	1	0	3
307	2	max	2341.634	2	8.04	4	0	1	0	1	0	1	.008	2
308		min	-2434.196	3	1.887	15	0	1	0	1	0	1	-.002	3
309	3	max	2341.464	2	7.171	4	0	1	0	1	0	1	.005	2
310		min	-2434.324	3	1.683	15	0	1	0	1	0	1	-.004	3
311	4	max	2341.294	2	6.302	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2434.452	3	1.479	15	0	1	0	1	0	1	-.005	3
313	5	max	2341.123	2	5.433	4	0	1	0	1	0	1	0	2
314		min	-2434.58	3	1.274	15	0	1	0	1	0	1	-.007	3
315	6	max	2340.953	2	4.564	4	0	1	0	1	0	1	-.002	15
316		min	-2434.707	3	1.07	15	0	1	0	1	0	1	-.008	3
317	7	max	2340.783	2	3.695	4	0	1	0	1	0	1	-.002	15
318		min	-2434.835	3	.866	15	0	1	0	1	0	1	-.009	4
319	8	max	2340.612	2	2.826	4	0	1	0	1	0	1	-.002	15
320		min	-2434.963	3	.662	15	0	1	0	1	0	1	-.01	4
321	9	max	2340.442	2	1.983	2	0	1	0	1	0	1	-.003	15
322		min	-2435.091	3	.378	12	0	1	0	1	0	1	-.011	4
323	10	max	2340.272	2	1.306	2	0	1	0	1	0	1	-.003	15
324		min	-2435.218	3	.022	3	0	1	0	1	0	1	-.012	4
325	11	max	2340.101	2	.629	2	0	1	0	1	0	1	-.003	15
326		min	-2435.346	3	-.486	3	0	1	0	1	0	1	-.012	4
327	12	max	2339.931	2	-.048	2	0	1	0	1	0	1	-.003	15
328		min	-2435.474	3	-.994	3	0	1	0	1	0	1	-.012	4
329	13	max	2339.761	2	-.359	15	0	1	0	1	0	1	-.003	15
330		min	-2435.602	3	-1.518	4	0	1	0	1	0	1	-.012	4
331	14	max	2339.59	2	-.564	15	0	1	0	1	0	1	-.003	15
332		min	-2435.73	3	-2.387	4	0	1	0	1	0	1	-.011	4
333	15	max	2339.42	2	-.768	15	0	1	0	1	0	1	-.002	15
334		min	-2435.857	3	-3.256	4	0	1	0	1	0	1	-.009	4
335	16	max	2339.25	2	-.972	15	0	1	0	1	0	1	-.002	15
336		min	-2435.985	3	-4.125	4	0	1	0	1	0	1	-.008	4
337	17	max	2339.079	2	-1.176	15	0	1	0	1	0	1	-.001	15
338		min	-2436.113	3	-4.994	4	0	1	0	1	0	1	-.006	4
339	18	max	2338.909	2	-1.381	15	0	1	0	1	0	1	0	15
340		min	-2436.241	3	-5.863	4	0	1	0	1	0	1	-.003	4
341	19	max	2338.739	2	-1.585	15	0	1	0	1	0	1	0	1
342		min	-2436.368	3	-6.732	4	0	1	0	1	0	1	0	1
343	M8	1	max	2865.07	1	0	1	0	1	0	1	0	1	1
344		min	-86.529	3	0	1	0	1	0	1	0	1	0	1
345	2	max	2865.24	1	0	1	0	1	0	1	0	1	0	1
346		min	-86.402	3	0	1	0	1	0	1	0	1	0	1
347	3	max	2865.411	1	0	1	0	1	0	1	0	1	0	1
348		min	-86.274	3	0	1	0	1	0	1	0	1	0	1
349	4	max	2865.581	1	0	1	0	1	0	1	0	1	0	1
350		min	-86.146	3	0	1	0	1	0	1	0	1	0	1
351	5	max	2865.751	1	0	1	0	1	0	1	0	1	0	1
352		min	-86.018	3	0	1	0	1	0	1	0	1	0	1
353	6	max	2865.922	1	0	1	0	1	0	1	0	1	0	1
354		min	-85.891	3	0	1	0	1	0	1	0	1	0	1
355	7	max	2866.092	1	0	1	0	1	0	1	0	1	0	1
356		min	-85.763	3	0	1	0	1	0	1	0	1	0	1
357	8	max	2866.262	1	0	1	0	1	0	1	0	1	0	1
358		min	-85.635	3	0	1	0	1	0	1	0	1	0	1
359	9	max	2866.433	1	0	1	0	1	0	1	0	1	0	1
360		min	-85.507	3	0	1	0	1	0	1	0	1	0	1
361	10	max	2866.603	1	0	1	0	1	0	1	0	1	0	1
362		min	-85.38	3	0	1	0	1	0	1	0	1	0	1
363	11	max	2866.773	1	0	1	0	1	0	1	0	1	0	1
364		min	-85.252	3	0	1	0	1	0	1	0	1	0	1
365	12	max	2866.944	1	0	1	0	1	0	1	0	1	0	1
366		min	-85.124	3	0	1	0	1	0	1	0	1	0	1
367	13	max	2867.114	1	0	1	0	1	0	1	0	1	0	1
368		min	-84.996	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2867.284	1	0	1	0	1	0	1	0	1	0	1
370			min	-84.869	3	0	1	0	1	0	1	0	1	0	1
371		15	max	2867.455	1	0	1	0	1	0	1	0	1	0	1
372			min	-84.741	3	0	1	0	1	0	1	0	1	0	1
373		16	max	2867.625	1	0	1	0	1	0	1	0	1	0	1
374			min	-84.613	3	0	1	0	1	0	1	0	1	0	1
375		17	max	2867.795	1	0	1	0	1	0	1	0	1	0	1
376			min	-84.485	3	0	1	0	1	0	1	0	1	0	1
377		18	max	2867.966	1	0	1	0	1	0	1	0	1	0	1
378			min	-84.357	3	0	1	0	1	0	1	0	1	0	1
379		19	max	2868.136	1	0	1	0	1	0	1	0	1	0	1
380			min	-84.23	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1010.382	1	2.022	4	-.028	15	0	1	0	1	0	1
382			min	-1195.944	3	.476	15	-.605	1	0	5	0	3	0	1
383		2	max	1010.911	1	1.951	4	-.028	15	0	1	0	15	0	15
384			min	-1195.547	3	.459	15	-.605	1	0	5	0	1	0	4
385		3	max	1011.441	1	1.88	4	-.028	15	0	1	0	15	0	15
386			min	-1195.15	3	.442	15	-.605	1	0	5	0	1	-.001	4
387		4	max	1011.97	1	1.809	4	-.028	15	0	1	0	15	0	15
388			min	-1194.753	3	.426	15	-.605	1	0	5	0	1	-.002	4
389		5	max	1012.499	1	1.738	4	-.028	15	0	1	0	15	0	15
390			min	-1194.356	3	.409	15	-.605	1	0	5	0	1	-.003	4
391		6	max	1013.028	1	1.667	4	-.028	15	0	1	0	15	0	15
392			min	-1193.959	3	.392	15	-.605	1	0	5	-.001	1	-.003	4
393		7	max	1013.558	1	1.596	4	-.028	15	0	1	0	15	0	15
394			min	-1193.562	3	.375	15	-.605	1	0	5	-.001	1	-.004	4
395		8	max	1014.087	1	1.525	4	-.028	15	0	1	0	15	-.001	15
396			min	-1193.166	3	.359	15	-.605	1	0	5	-.002	1	-.004	4
397		9	max	1014.616	1	1.454	4	-.028	15	0	1	0	15	-.001	15
398			min	-1192.769	3	.342	15	-.605	1	0	5	-.002	1	-.005	4
399		10	max	1015.146	1	1.383	4	-.028	15	0	1	0	15	-.001	15
400			min	-1192.372	3	.325	15	-.605	1	0	5	-.002	1	-.005	4
401		11	max	1015.675	1	1.312	4	-.028	15	0	1	0	15	-.001	15
402			min	-1191.975	3	.309	15	-.605	1	0	5	-.002	1	-.006	4
403		12	max	1016.204	1	1.241	4	-.028	15	0	1	0	15	-.002	15
404			min	-1191.578	3	.292	15	-.605	1	0	5	-.002	1	-.006	4
405		13	max	1016.733	1	1.17	4	-.028	15	0	1	0	15	-.002	15
406			min	-1191.181	3	.275	15	-.605	1	0	5	-.003	1	-.007	4
407		14	max	1017.263	1	1.099	4	-.028	15	0	1	0	15	-.002	15
408			min	-1190.784	3	.259	15	-.605	1	0	5	-.003	1	-.007	4
409		15	max	1017.792	1	1.028	4	-.028	15	0	1	0	15	-.002	15
410			min	-1190.387	3	.242	15	-.605	1	0	5	-.003	1	-.008	4
411		16	max	1018.321	1	.957	4	-.028	15	0	1	0	15	-.002	15
412			min	-1189.99	3	.219	12	-.605	1	0	5	-.003	1	-.008	4
413		17	max	1018.851	1	.886	4	-.028	15	0	1	0	15	-.002	15
414			min	-1189.593	3	.191	12	-.605	1	0	5	-.003	1	-.008	4
415		18	max	1019.38	1	.815	4	-.028	15	0	1	0	15	-.002	15
416			min	-1189.196	3	.163	12	-.605	1	0	5	-.004	1	-.009	4
417		19	max	1019.909	1	.758	2	-.028	15	0	1	0	15	-.002	15
418			min	-1188.799	3	.136	12	-.605	1	0	5	-.004	1	-.009	4
419	M11	1	max	624.614	2	8.874	4	-.022	15	0	1	0	15	.009	4
420			min	-785.835	3	2.086	15	-.472	1	0	15	0	1	.002	15
421		2	max	624.444	2	8.005	4	-.022	15	0	1	0	15	.005	4
422			min	-785.962	3	1.882	15	-.472	1	0	15	0	1	0	12
423		3	max	624.273	2	7.136	4	-.022	15	0	1	0	15	.002	2
424			min	-786.09	3	1.678	15	-.472	1	0	15	0	1	0	3
425		4	max	624.103	2	6.267	4	-.022	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-786.218	3	1.473	15	-.472	1	0	15	-.001	1	-.002	3
427		5	max	623.933	2	5.398	4	-.022	15	0	1	0	15	-.001	15
428			min	-786.346	3	1.269	15	-.472	1	0	15	-.001	1	-.005	4
429		6	max	623.762	2	4.53	4	-.022	15	0	1	0	15	-.002	15
430			min	-786.473	3	1.065	15	-.472	1	0	15	-.002	1	-.007	4
431		7	max	623.592	2	3.661	4	-.022	15	0	1	0	15	-.002	15
432			min	-786.601	3	.861	15	-.472	1	0	15	-.002	1	-.009	4
433		8	max	623.422	2	2.792	4	-.022	15	0	1	0	15	-.002	15
434			min	-786.729	3	.656	15	-.472	1	0	15	-.002	1	-.01	4
435		9	max	623.251	2	1.923	4	-.022	15	0	1	0	15	-.003	15
436			min	-786.857	3	.452	15	-.472	1	0	15	-.002	1	-.011	4
437		10	max	623.081	2	1.054	4	-.022	15	0	1	0	15	-.003	15
438			min	-786.984	3	.248	15	-.472	1	0	15	-.002	1	-.012	4
439		11	max	622.911	2	.281	2	-.022	15	0	1	0	15	-.003	15
440			min	-787.112	3	-.08	3	-.472	1	0	15	-.003	1	-.012	4
441		12	max	622.74	2	-.16	15	-.022	15	0	1	0	15	-.003	15
442			min	-787.24	3	-.684	4	-.472	1	0	15	-.003	1	-.012	4
443		13	max	622.57	2	-.365	15	-.022	15	0	1	0	15	-.003	15
444			min	-787.368	3	-1.553	4	-.472	1	0	15	-.003	1	-.012	4
445		14	max	622.4	2	-.569	15	-.022	15	0	1	0	15	-.003	15
446			min	-787.495	3	-2.422	4	-.472	1	0	15	-.003	1	-.011	4
447		15	max	622.229	2	-.773	15	-.022	15	0	1	0	15	-.002	15
448			min	-787.623	3	-3.29	4	-.472	1	0	15	-.004	1	-.009	4
449		16	max	622.059	2	-.977	15	-.022	15	0	1	0	15	-.002	15
450			min	-787.751	3	-4.159	4	-.472	1	0	15	-.004	1	-.008	4
451		17	max	621.888	2	-1.182	15	-.022	15	0	1	0	15	-.001	15
452			min	-787.879	3	-5.028	4	-.472	1	0	15	-.004	1	-.006	4
453		18	max	621.718	2	-1.386	15	-.022	15	0	1	0	15	0	15
454			min	-788.006	3	-5.897	4	-.472	1	0	15	-.004	1	-.003	4
455		19	max	621.548	2	-1.59	15	-.022	15	0	1	0	15	0	1
456			min	-788.134	3	-6.766	4	-.472	1	0	15	-.004	1	0	1
457	M12	1	max	1120.407	1	0	1	19.321	1	0	1	0	15	0	1
458			min	3.982	3	0	1	.899	15	0	1	-.004	1	0	1
459		2	max	1120.577	1	0	1	19.321	1	0	1	0	15	0	1
460			min	4.11	3	0	1	.899	15	0	1	-.001	1	0	1
461		3	max	1120.748	1	0	1	19.321	1	0	1	0	1	0	1
462			min	4.238	3	0	1	.899	15	0	1	0	15	0	1
463		4	max	1120.918	1	0	1	19.321	1	0	1	.003	1	0	1
464			min	4.366	3	0	1	.899	15	0	1	0	15	0	1
465		5	max	1121.088	1	0	1	19.321	1	0	1	.005	1	0	1
466			min	4.493	3	0	1	.899	15	0	1	0	15	0	1
467		6	max	1121.259	1	0	1	19.321	1	0	1	.008	1	0	1
468			min	4.621	3	0	1	.899	15	0	1	0	15	0	1
469		7	max	1121.429	1	0	1	19.321	1	0	1	.01	1	0	1
470			min	4.749	3	0	1	.899	15	0	1	0	15	0	1
471		8	max	1121.599	1	0	1	19.321	1	0	1	.012	1	0	1
472			min	4.877	3	0	1	.899	15	0	1	0	15	0	1
473		9	max	1121.77	1	0	1	19.321	1	0	1	.014	1	0	1
474			min	5.004	3	0	1	.899	15	0	1	0	15	0	1
475		10	max	1121.94	1	0	1	19.321	1	0	1	.016	1	0	1
476			min	5.132	3	0	1	.899	15	0	1	0	15	0	1
477		11	max	1122.11	1	0	1	19.321	1	0	1	.019	1	0	1
478			min	5.26	3	0	1	.899	15	0	1	0	15	0	1
479		12	max	1122.281	1	0	1	19.321	1	0	1	.021	1	0	1
480			min	5.388	3	0	1	.899	15	0	1	0	15	0	1
481		13	max	1122.451	1	0	1	19.321	1	0	1	.023	1	0	1
482			min	5.516	3	0	1	.899	15	0	1	.001	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1122.621	1	0	1	19.321	1	0	1	.025	1	0	1
484			min	5.643	3	0	1	.899	15	0	1	.001	15	0	1
485		15	max	1122.792	1	0	1	19.321	1	0	1	.028	1	0	1
486			min	5.771	3	0	1	.899	15	0	1	.001	15	0	1
487		16	max	1122.962	1	0	1	19.321	1	0	1	.03	1	0	1
488			min	5.899	3	0	1	.899	15	0	1	.001	15	0	1
489		17	max	1123.132	1	0	1	19.321	1	0	1	.032	1	0	1
490			min	6.027	3	0	1	.899	15	0	1	.001	15	0	1
491		18	max	1123.303	1	0	1	19.321	1	0	1	.034	1	0	1
492			min	6.154	3	0	1	.899	15	0	1	.002	15	0	1
493		19	max	1123.473	1	0	1	19.321	1	0	1	.036	1	0	1
494			min	6.282	3	0	1	.899	15	0	1	.002	15	0	1
495	M1	1	max	211.519	1	630.773	3	-5.05	15	0	1	.304	1	0	3
496			min	9.83	15	-422.592	1	-107.869	1	0	3	.014	15	-.015	2
497		2	max	212.362	1	629.679	3	-5.05	15	0	1	.237	1	.249	1
498			min	10.084	15	-424.051	1	-107.869	1	0	3	.011	15	-.391	3
499		3	max	505.239	3	492.925	1	-5.022	15	0	3	.17	1	.502	1
500			min	-308.778	2	-467.367	3	-107.597	1	0	1	.008	15	-.77	3
501		4	max	505.871	3	491.466	1	-5.022	15	0	3	.103	1	.197	1
502			min	-307.936	2	-468.461	3	-107.597	1	0	1	.005	15	-.479	3
503		5	max	506.503	3	490.007	1	-5.022	15	0	3	.036	1	-.005	15
504			min	-307.093	2	-469.555	3	-107.597	1	0	1	.002	15	-.188	3
505		6	max	507.134	3	488.548	1	-5.022	15	0	3	-.001	15	.104	3
506			min	-306.251	2	-470.65	3	-107.597	1	0	1	-.031	1	-.433	2
507		7	max	507.766	3	487.089	1	-5.022	15	0	3	-.005	15	.396	3
508			min	-305.408	2	-471.744	3	-107.597	1	0	1	-.097	1	-.733	2
509		8	max	508.398	3	485.63	1	-5.022	15	0	3	-.008	15	.689	3
510			min	-304.566	2	-472.838	3	-107.597	1	0	1	-.164	1	-1.032	2
511		9	max	525.877	3	44.507	2	-7.768	15	0	9	.101	1	.805	3
512			min	-212.929	2	.446	15	-166.265	1	0	3	.005	15	-1.181	2
513		10	max	526.508	3	43.048	2	-7.768	15	0	9	0	15	.785	3
514			min	-212.086	2	.006	15	-166.265	1	0	3	-.002	1	-1.209	2
515		11	max	527.14	3	41.589	2	-7.768	15	0	9	-.005	15	.766	3
516			min	-211.244	2	-1.733	4	-166.265	1	0	3	-.105	1	-1.235	2
517		12	max	544.5	3	311.508	3	-4.822	15	0	2	.161	1	.669	3
518			min	-125.271	10	-573.573	2	-103.493	1	0	3	.007	15	-1.094	2
519		13	max	545.132	3	310.414	3	-4.822	15	0	2	.096	1	.476	3
520			min	-124.569	10	-575.033	2	-103.493	1	0	3	.004	15	-.738	2
521		14	max	545.764	3	309.319	3	-4.822	15	0	2	.032	1	.283	3
522			min	-123.867	10	-576.492	2	-103.493	1	0	3	.002	15	-.385	1
523		15	max	546.395	3	308.225	3	-4.822	15	0	2	-.001	15	.092	3
524			min	-123.165	10	-577.951	2	-103.493	1	0	3	-.032	1	-.051	1
525		16	max	547.027	3	307.131	3	-4.822	15	0	2	-.004	15	.337	2
526			min	-122.463	10	-579.41	2	-103.493	1	0	3	-.096	1	-.099	3
527		17	max	547.659	3	306.036	3	-4.822	15	0	2	-.007	15	.697	2
528			min	-121.761	10	-580.869	2	-103.493	1	0	3	-.16	1	-.29	3
529		18	max	-10.101	15	580.234	2	-5.698	15	0	3	-.011	15	.349	2
530			min	-212.836	1	-247.946	3	-121.853	1	0	2	-.231	1	-.142	3
531		19	max	-9.847	15	578.775	2	-5.698	15	0	3	-.014	15	.012	3
532			min	-211.994	1	-249.04	3	-121.853	1	0	2	-.307	1	-.012	1
533	M5	1	max	462.983	1	2101.231	3	0	1	0	1	0	1	.03	2
534			min	20.098	12	-1440.176	1	0	1	0	1	0	1	0	3
535		2	max	463.825	1	2100.136	3	0	1	0	1	0	1	.922	1
536			min	20.519	12	-1441.635	1	0	1	0	1	0	1	-1.304	3
537		3	max	1603.788	3	1440.629	1	0	1	0	1	0	1	1.786	1
538			min	-1065.51	2	-1467.071	3	0	1	0	1	0	1	-2.568	3
539		4	max	1604.42	3	1439.17	1	0	1	0	1	0	1	.892	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1064.667	2	-1468.165	3	0	1	0	1	0	1	-1.657	3
541		5	max	1605.052	3	1437.711	1	0	1	0	1	0	1	.024	9
542			min	-1063.825	2	-1469.26	3	0	1	0	1	0	1	-.745	3
543		6	max	1605.683	3	1436.252	1	0	1	0	1	0	1	.167	3
544			min	-1062.982	2	-1470.354	3	0	1	0	1	0	1	-.939	2
545		7	max	1606.315	3	1434.793	1	0	1	0	1	0	1	1.08	3
546			min	-1062.14	2	-1471.448	3	0	1	0	1	0	1	-1.817	2
547		8	max	1606.947	3	1433.334	1	0	1	0	1	0	1	1.993	3
548			min	-1061.298	2	-1472.543	3	0	1	0	1	0	1	-2.695	2
549		9	max	1636.508	3	149.056	2	0	1	0	1	0	1	2.295	3
550			min	-871.55	2	.445	15	0	1	0	1	0	1	-3.076	2
551		10	max	1637.14	3	147.597	2	0	1	0	1	0	1	2.223	3
552			min	-870.707	2	.005	15	0	1	0	1	0	1	-3.168	2
553		11	max	1637.772	3	146.138	2	0	1	0	1	0	1	2.152	3
554			min	-869.865	2	-1.459	4	0	1	0	1	0	1	-3.259	2
555		12	max	1667.571	3	957.736	3	0	1	0	1	0	1	1.889	3
556			min	-680.143	2	-1713.882	2	0	1	0	1	0	1	-2.914	2
557		13	max	1668.202	3	956.641	3	0	1	0	1	0	1	1.295	3
558			min	-679.3	2	-1715.341	2	0	1	0	1	0	1	-1.85	2
559		14	max	1668.834	3	955.547	3	0	1	0	1	0	1	.702	3
560			min	-678.458	2	-1716.8	2	0	1	0	1	0	1	-.812	1
561		15	max	1669.466	3	954.453	3	0	1	0	1	0	1	.281	2
562			min	-677.616	2	-1718.259	2	0	1	0	1	0	1	0	13
563		16	max	1670.098	3	953.359	3	0	1	0	1	0	1	1.348	2
564			min	-676.773	2	-1719.718	2	0	1	0	1	0	1	-.483	3
565		17	max	1670.73	3	952.264	3	0	1	0	1	0	1	2.416	2
566			min	-675.931	2	-1721.177	2	0	1	0	1	0	1	-1.074	3
567		18	max	-20.969	12	1961.981	2	0	1	0	1	0	1	1.238	2
568			min	-462.877	1	-861.949	3	0	1	0	1	0	1	-.56	3
569		19	max	-20.547	12	1960.522	2	0	1	0	1	0	1	.023	1
570			min	-462.034	1	-863.043	3	0	1	0	1	0	1	-.024	3
571	M9	1	max	211.519	1	630.773	3	107.869	1	0	3	-.014	15	0	3
572			min	9.83	15	-422.592	1	5.05	15	0	1	-.304	1	-.015	2
573		2	max	212.362	1	629.679	3	107.869	1	0	3	-.011	15	.249	1
574			min	10.084	15	-424.051	1	5.05	15	0	1	-.237	1	-.391	3
575		3	max	505.239	3	492.925	1	107.597	1	0	1	-.008	15	.502	1
576			min	-308.778	2	-467.367	3	5.022	15	0	3	-.17	1	-.77	3
577		4	max	505.871	3	491.466	1	107.597	1	0	1	-.005	15	.197	1
578			min	-307.936	2	-468.461	3	5.022	15	0	3	-.103	1	-.479	3
579		5	max	506.503	3	490.007	1	107.597	1	0	1	-.002	15	-.005	15
580			min	-307.093	2	-469.555	3	5.022	15	0	3	-.036	1	-.188	3
581		6	max	507.134	3	488.548	1	107.597	1	0	1	.031	1	.104	3
582			min	-306.251	2	-470.65	3	5.022	15	0	3	.001	15	-.433	2
583		7	max	507.766	3	487.089	1	107.597	1	0	1	.097	1	.396	3
584			min	-305.408	2	-471.744	3	5.022	15	0	3	.005	15	-.733	2
585		8	max	508.398	3	485.63	1	107.597	1	0	1	.164	1	.689	3
586			min	-304.566	2	-472.838	3	5.022	15	0	3	.008	15	-1.032	2
587		9	max	525.877	3	44.507	2	166.265	1	0	3	-.005	15	.805	3
588			min	-212.929	2	.446	15	7.768	15	0	9	-.101	1	-1.181	2
589		10	max	526.508	3	43.048	2	166.265	1	0	3	.002	1	.785	3
590			min	-212.086	2	.006	15	7.768	15	0	9	0	15	-1.209	2
591		11	max	527.14	3	41.589	2	166.265	1	0	3	.105	1	.766	3
592			min	-211.244	2	-1.733	4	7.768	15	0	9	.005	15	-1.235	2
593		12	max	544.5	3	311.508	3	103.493	1	0	3	-.007	15	.669	3
594			min	-125.271	10	-573.573	2	4.822	15	0	2	-.161	1	-1.094	2
595		13	max	545.132	3	310.414	3	103.493	1	0	3	-.004	15	.476	3
596			min	-124.569	10	-575.033	2	4.822	15	0	2	-.096	1	-.738	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	545.764	3	309.319	3	103.493	1	0	3	-.002	15	.283	3
598		min	-123.867	10	-576.492	2	4.822	15	0	2	-.032	1	-.385	1
599	15	max	546.395	3	308.225	3	103.493	1	0	3	.032	1	.092	3
600		min	-123.165	10	-577.951	2	4.822	15	0	2	.001	15	-.051	1
601	16	max	547.027	3	307.131	3	103.493	1	0	3	.096	1	.337	2
602		min	-122.463	10	-579.41	2	4.822	15	0	2	.004	15	-.099	3
603	17	max	547.659	3	306.036	3	103.493	1	0	3	.16	1	.697	2
604		min	-121.761	10	-580.869	2	4.822	15	0	2	.007	15	-.29	3
605	18	max	-10.101	15	580.234	2	121.853	1	0	2	.231	1	.349	2
606		min	-212.836	1	-247.946	3	5.698	15	0	3	.011	15	-.142	3
607	19	max	-9.847	15	578.775	2	121.853	1	0	2	.307	1	.012	3
608		min	-211.994	1	-249.04	3	5.698	15	0	3	.014	15	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.176	2	.01	3	1.213e-2	2	NC	1	NC	1
2			min	0	15	-.035	3	-.005	2	-2.475e-3	3	NC	1	NC	1
3		2	max	0	1	.264	3	.051	1	1.36e-2	2	NC	5	NC	2
4			min	0	15	-.01	9	.002	10	-2.473e-3	3	803.159	3	4782.15	1
5		3	max	0	1	.506	3	.121	1	1.508e-2	2	NC	5	NC	3
6			min	0	15	-.142	1	.006	15	-2.471e-3	3	443.757	3	1997.315	1
7		4	max	0	1	.653	3	.181	1	1.655e-2	2	NC	5	NC	3
8			min	0	15	-.214	1	.009	15	-2.469e-3	3	348.894	3	1334.734	1
9		5	max	0	1	.687	3	.211	1	1.802e-2	2	NC	5	NC	5
10			min	0	15	-.212	1	.01	15	-2.467e-3	3	332.341	3	1144.005	1
11		6	max	0	1	.611	3	.203	1	1.949e-2	2	NC	5	NC	5
12			min	0	15	-.137	1	.01	15	-2.465e-3	3	371.438	3	1190.975	1
13		7	max	0	1	.448	3	.158	1	2.096e-2	2	NC	5	NC	5
14			min	0	15	-.016	9	.008	15	-2.463e-3	3	497.202	3	1526.955	1
15		8	max	0	1	.24	3	.091	1	2.243e-2	2	NC	1	NC	5
16			min	0	15	.005	15	0	10	-2.461e-3	3	872.621	3	2674.933	1
17		9	max	0	1	.311	2	.032	3	2.39e-2	2	NC	4	NC	1
18			min	0	15	.009	15	-.009	10	-2.459e-3	3	1784.908	2	NC	1
19		10	max	0	1	.369	2	.03	3	2.537e-2	2	NC	3	NC	1
20			min	0	1	-.034	3	-.021	2	-2.457e-3	3	1242.86	2	NC	1
21		11	max	0	15	.311	2	.032	3	2.39e-2	2	NC	4	NC	1
22			min	0	1	.009	15	-.009	10	-2.459e-3	3	1784.908	2	NC	1
23		12	max	0	15	.24	3	.091	1	2.243e-2	2	NC	1	NC	5
24			min	0	1	.005	15	0	10	-2.461e-3	3	872.621	3	2674.933	1
25		13	max	0	15	.448	3	.158	1	2.096e-2	2	NC	5	NC	5
26			min	0	1	-.016	9	.008	15	-2.463e-3	3	497.202	3	1526.955	1
27		14	max	0	15	.611	3	.203	1	1.949e-2	2	NC	5	NC	5
28			min	0	1	-.137	1	.01	15	-2.465e-3	3	371.438	3	1190.975	1
29		15	max	0	15	.687	3	.211	1	1.802e-2	2	NC	5	NC	5
30			min	0	1	-.212	1	.01	15	-2.467e-3	3	332.341	3	1144.005	1
31		16	max	0	15	.653	3	.181	1	1.655e-2	2	NC	5	NC	3
32			min	0	1	-.214	1	.009	15	-2.469e-3	3	348.894	3	1334.734	1
33		17	max	0	15	.506	3	.121	1	1.508e-2	2	NC	5	NC	3
34			min	0	1	-.142	1	.006	15	-2.471e-3	3	443.757	3	1997.315	1
35		18	max	0	15	.264	3	.051	1	1.36e-2	2	NC	5	NC	2
36			min	0	1	-.01	9	.002	10	-2.473e-3	3	803.159	3	4782.15	1
37		19	max	0	15	.176	2	.01	3	1.213e-2	2	NC	1	NC	1
38			min	-.001	1	-.035	3	-.005	2	-2.475e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.333	3	.009	3	7.027e-3	2	NC	1	NC	1
40			min	0	15	-.544	2	-.005	2	-5.037e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.657	3	.034	1	8.29e-3	2	NC	5	NC	2
42		min	0	15	-.876	2	0	10	-6.06e-3	3	706.968	1	7326.386	1
43	3	max	0	1	.936	3	.095	1	9.553e-3	2	NC	15	NC	3
44		min	0	15	-1.167	2	.005	15	-7.083e-3	3	376.929	1	2565.313	1
45	4	max	0	1	1.137	3	.152	1	1.082e-2	2	9324.703	15	NC	3
46		min	0	15	-1.39	2	.007	15	-8.107e-3	3	278.335	1	1593.366	1
47	5	max	0	1	1.245	3	.184	1	1.208e-2	2	8068.431	15	NC	3
48		min	0	15	-1.53	2	.009	15	-9.13e-3	3	239.862	1	1311.742	1
49	6	max	0	1	1.26	3	.182	1	1.334e-2	2	7732.466	15	NC	3
50		min	0	15	-1.585	2	.009	15	-1.015e-2	3	228.569	1	1330.857	1
51	7	max	0	1	1.196	3	.145	1	1.46e-2	2	7999.146	15	NC	3
52		min	0	15	-1.566	2	.007	15	-1.118e-2	3	234.63	1	1675.133	1
53	8	max	0	1	1.085	3	.084	1	1.587e-2	2	8740.161	15	NC	3
54		min	0	15	-1.497	2	0	10	-1.22e-2	3	251.824	2	2889.077	1
55	9	max	0	1	.971	3	.028	3	1.713e-2	2	9734.283	15	NC	1
56		min	0	15	-1.418	2	-.008	10	-1.322e-2	3	274.794	2	NC	1
57	10	max	0	1	.916	3	.027	3	1.839e-2	2	NC	15	NC	1
58		min	0	1	-1.377	2	-.019	2	-1.425e-2	3	288.07	2	NC	1
59	11	max	0	15	.971	3	.028	3	1.713e-2	2	9734.283	15	NC	1
60		min	0	1	-1.418	2	-.008	10	-1.322e-2	3	274.794	2	NC	1
61	12	max	0	15	1.085	3	.084	1	1.587e-2	2	8740.161	15	NC	3
62		min	0	1	-1.497	2	0	10	-1.22e-2	3	251.824	2	2889.077	1
63	13	max	0	15	1.196	3	.145	1	1.46e-2	2	7999.146	15	NC	3
64		min	0	1	-1.566	2	.007	15	-1.118e-2	3	234.63	1	1675.133	1
65	14	max	0	15	1.26	3	.182	1	1.334e-2	2	7732.466	15	NC	3
66		min	0	1	-1.585	2	.009	15	-1.015e-2	3	228.569	1	1330.857	1
67	15	max	0	15	1.245	3	.184	1	1.208e-2	2	8068.431	15	NC	3
68		min	0	1	-1.53	2	.009	15	-9.13e-3	3	239.862	1	1311.742	1
69	16	max	0	15	1.137	3	.152	1	1.082e-2	2	9324.703	15	NC	3
70		min	0	1	-1.39	2	.007	15	-8.107e-3	3	278.335	1	1593.366	1
71	17	max	0	15	.936	3	.095	1	9.553e-3	2	NC	15	NC	3
72		min	0	1	-1.167	2	.005	15	-7.083e-3	3	376.929	1	2565.313	1
73	18	max	0	15	.657	3	.034	1	8.29e-3	2	NC	5	NC	2
74		min	0	1	-.876	2	0	10	-6.06e-3	3	706.968	1	7326.386	1
75	19	max	0	15	.333	3	.009	3	7.027e-3	2	NC	1	NC	1
76		min	0	1	-.544	2	-.005	2	-5.037e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.34	.008	3	4.285e-3	3	NC	1	NC	1
78		min	0	1	-.543	2	-.004	2	-7.31e-3	2	NC	1	NC	1
79	2	max	0	15	.56	3	.034	1	5.158e-3	3	NC	5	NC	2
80		min	0	1	-.957	2	0	10	-8.63e-3	2	579.209	2	7263.65	1
81	3	max	0	15	.754	3	.095	1	6.03e-3	3	NC	15	NC	3
82		min	0	1	-1.316	2	.005	15	-9.95e-3	2	310.583	2	2552.455	1
83	4	max	0	15	.905	3	.153	1	6.903e-3	3	9340.497	15	NC	3
84		min	0	1	-1.58	2	.007	15	-1.127e-2	2	231.567	2	1587.239	1
85	5	max	0	15	1.003	3	.185	1	7.775e-3	3	8083.729	15	NC	3
86		min	0	1	-1.729	2	.009	15	-1.259e-2	2	202.449	2	1307.244	1
87	6	max	0	15	1.046	3	.182	1	8.648e-3	3	7749.339	15	NC	3
88		min	0	1	-1.762	2	.009	15	-1.391e-2	2	196.907	2	1326.206	1
89	7	max	0	15	1.042	3	.145	1	9.52e-3	3	8019.73	15	NC	3
90		min	0	1	-1.697	2	.007	15	-1.523e-2	2	207.928	2	1668.078	1
91	8	max	0	15	1.005	3	.085	1	1.039e-2	3	8767.045	15	NC	3
92		min	0	1	-1.571	2	0	10	-1.655e-2	2	233.413	2	2869.498	1
93	9	max	0	15	.959	3	.026	3	1.127e-2	3	9769.47	15	NC	1
94		min	0	1	-1.439	2	-.008	10	-1.787e-2	2	267.817	2	NC	1
95	10	max	0	1	.936	3	.025	3	1.214e-2	3	NC	15	NC	1
96		min	0	1	-1.375	2	-.018	2	-1.919e-2	2	288.399	2	NC	1
97	11	max	0	1	.959	3	.026	3	1.127e-2	3	9769.47	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.439	2	-.008	10	-1.787e-2	2	267.817	2	NC	1
99		max	0	1	1.005	3	.085	1	1.039e-2	3	8767.045	15	NC	3
100		min	0	15	-1.571	2	0	10	-1.655e-2	2	233.413	2	2869.498	1
101		max	0	1	1.042	3	.145	1	9.52e-3	3	8019.73	15	NC	3
102		min	0	15	-1.697	2	.007	15	-1.523e-2	2	207.928	2	1668.078	1
103		max	0	1	1.046	3	.182	1	8.648e-3	3	7749.339	15	NC	3
104		min	0	15	-1.762	2	.009	15	-1.391e-2	2	196.907	2	1326.206	1
105		max	0	1	1.003	3	.185	1	7.775e-3	3	8083.729	15	NC	3
106		min	0	15	-1.729	2	.009	15	-1.259e-2	2	202.449	2	1307.244	1
107		max	0	1	.905	3	.153	1	6.903e-3	3	9340.497	15	NC	3
108		min	0	15	-1.58	2	.007	15	-1.127e-2	2	231.567	2	1587.239	1
109		max	0	1	.754	3	.095	1	6.03e-3	3	NC	15	NC	3
110		min	0	15	-1.316	2	.005	15	-9.95e-3	2	310.583	2	2552.455	1
111		max	0	1	.56	3	.034	1	5.158e-3	3	NC	5	NC	2
112		min	0	15	-.957	2	0	10	-8.63e-3	2	579.209	2	7263.65	1
113		max	0	1	.34	3	.008	3	4.285e-3	3	NC	1	NC	1
114		min	0	15	-.543	2	-.004	2	-7.31e-3	2	NC	1	NC	1
115	M16	max	0	15	.161	1	.007	3	7.775e-3	3	NC	1	NC	1
116		min	-.001	1	-.115	3	-.004	2	-1.045e-2	1	NC	1	NC	1
117		max	0	15	.003	13	.051	1	8.967e-3	3	NC	5	NC	2
118		min	-.001	1	-.094	2	.003	15	-1.158e-2	1	956.741	2	4846.638	1
119		max	0	15	.05	3	.12	1	1.016e-2	3	NC	5	NC	3
120		min	0	1	-.293	2	.006	15	-1.271e-2	1	533.597	2	2011.648	1
121		max	0	15	.084	3	.18	1	1.135e-2	3	NC	5	NC	3
122		min	0	1	-.405	2	.009	15	-1.384e-2	1	426.967	2	1339.743	1
123		max	0	15	.074	3	.211	1	1.254e-2	3	NC	5	NC	3
124		min	0	1	-.415	2	.01	15	-1.497e-2	1	419.6	2	1145.222	1
125		max	0	15	.023	3	.203	1	1.373e-2	3	NC	5	NC	3
126		min	0	1	-.326	2	.01	15	-1.61e-2	1	497.568	2	1188.705	1
127		max	0	15	.002	13	.159	1	1.493e-2	3	NC	5	NC	3
128		min	0	1	-.158	2	.008	15	-1.723e-2	1	761.894	2	1516.878	1
129		max	0	15	.093	1	.092	1	1.612e-2	3	NC	4	NC	3
130		min	0	1	-.157	3	.003	10	-1.836e-2	1	2171.012	2	2626.691	1
131		max	0	15	.257	1	.026	1	1.731e-2	3	NC	4	NC	2
132		min	0	1	-.242	3	-.006	10	-1.949e-2	1	1891.604	3	9614.634	1
133		max	0	1	.33	1	.021	3	1.85e-2	3	NC	5	NC	1
134		min	0	1	-.279	3	-.016	2	-2.062e-2	1	1425.405	1	NC	1
135		max	0	1	.257	1	.026	1	1.731e-2	3	NC	4	NC	2
136		min	0	15	-.242	3	-.006	10	-1.949e-2	1	1891.604	3	9614.634	1
137		max	0	1	.093	1	.092	1	1.612e-2	3	NC	4	NC	3
138		min	0	15	-.157	3	.003	10	-1.836e-2	1	2171.012	2	2626.691	1
139		max	0	1	.002	13	.159	1	1.493e-2	3	NC	5	NC	3
140		min	0	15	-.158	2	.008	15	-1.723e-2	1	761.894	2	1516.878	1
141		max	0	1	.023	3	.203	1	1.373e-2	3	NC	5	NC	3
142		min	0	15	-.326	2	.01	15	-1.61e-2	1	497.568	2	1188.705	1
143		max	0	1	.074	3	.211	1	1.254e-2	3	NC	5	NC	3
144		min	0	15	-.415	2	.01	15	-1.497e-2	1	419.6	2	1145.222	1
145		max	0	1	.084	3	.18	1	1.135e-2	3	NC	5	NC	3
146		min	0	15	-.405	2	.009	15	-1.384e-2	1	426.967	2	1339.743	1
147		max	0	1	.05	3	.12	1	1.016e-2	3	NC	5	NC	3
148		min	0	15	-.293	2	.006	15	-1.271e-2	1	533.597	2	2011.648	1
149		max	.001	1	.003	13	.051	1	8.967e-3	3	NC	5	NC	2
150		min	0	15	-.094	2	.003	15	-1.158e-2	1	956.741	2	4846.638	1
151		max	.001	1	.161	1	.007	3	7.775e-3	3	NC	1	NC	1
152		min	0	15	-.115	3	-.004	2	-1.045e-2	1	NC	1	NC	1
153	M2	max	.008	1	.009	2	.014	1	-1.553e-5	15	NC	1	NC	2
154		min	-.009	3	-.015	3	0	15	-3.33e-4	1	8686.98	2	5469.957	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.007	1	.007	2	.013	1	-1.474e-5	15	NC	1	NC	2
156			min	-.008	3	-.014	3	0	15	-3.161e-4	1	NC	1	5961.094	1
157		3	max	.007	1	.006	2	.012	1	-1.395e-5	15	NC	1	NC	2
158			min	-.008	3	-.014	3	0	15	-2.992e-4	1	NC	1	6545.426	1
159		4	max	.006	1	.005	2	.011	1	-1.317e-5	15	NC	1	NC	2
160			min	-.007	3	-.014	3	0	15	-2.823e-4	1	NC	1	7247.383	1
161		5	max	.006	1	.003	2	.01	1	-1.238e-5	15	NC	1	NC	2
162			min	-.007	3	-.013	3	0	15	-2.653e-4	1	NC	1	8100.205	1
163		6	max	.005	1	.002	2	.008	1	-1.159e-5	15	NC	1	NC	2
164			min	-.006	3	-.013	3	0	15	-2.484e-4	1	NC	1	9150.058	1
165		7	max	.005	1	0	2	.007	1	-1.08e-5	15	NC	1	NC	1
166			min	-.006	3	-.012	3	0	15	-2.315e-4	1	NC	1	NC	1
167		8	max	.005	1	0	2	.006	1	-1.001e-5	15	NC	1	NC	1
168			min	-.005	3	-.012	3	0	15	-2.146e-4	1	NC	1	NC	1
169		9	max	.004	1	-.001	2	.005	1	-9.226e-6	15	NC	1	NC	1
170			min	-.005	3	-.011	3	0	15	-1.977e-4	1	NC	1	NC	1
171		10	max	.004	1	-.002	15	.005	1	-8.438e-6	15	NC	1	NC	1
172			min	-.004	3	-.01	3	0	15	-1.808e-4	1	NC	1	NC	1
173		11	max	.003	1	-.002	15	.004	1	-7.65e-6	15	NC	1	NC	1
174			min	-.004	3	-.01	3	0	15	-1.639e-4	1	NC	1	NC	1
175		12	max	.003	1	-.002	15	.003	1	-6.863e-6	15	NC	1	NC	1
176			min	-.003	3	-.009	3	0	15	-1.47e-4	1	NC	1	NC	1
177		13	max	.003	1	-.002	15	.002	1	-6.075e-6	15	NC	1	NC	1
178			min	-.003	3	-.008	3	0	15	-1.3e-4	1	NC	1	NC	1
179		14	max	.002	1	-.002	15	.002	1	-5.287e-6	15	NC	1	NC	1
180			min	-.002	3	-.007	3	0	15	-1.131e-4	1	NC	1	NC	1
181		15	max	.002	1	-.001	15	.001	1	-4.499e-6	15	NC	1	NC	1
182			min	-.002	3	-.006	4	0	15	-9.622e-5	1	NC	1	NC	1
183		16	max	.001	1	-.001	15	0	1	-3.711e-6	15	NC	1	NC	1
184			min	-.001	3	-.005	4	0	15	-7.93e-5	1	NC	1	NC	1
185		17	max	0	1	0	15	0	1	-2.923e-6	15	NC	1	NC	1
186			min	0	3	-.003	4	0	15	-6.239e-5	1	NC	1	NC	1
187		18	max	0	1	0	15	0	1	-2.135e-6	15	NC	1	NC	1
188			min	0	3	-.002	4	0	15	-4.548e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-1.347e-6	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-2.856e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	5.413e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	2.561e-7	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	4.177e-5	1	NC	1	NC	1
194			min	0	2	-.003	4	0	1	1.947e-6	15	NC	1	NC	1
195		3	max	0	3	-.001	15	0	15	7.813e-5	1	NC	1	NC	1
196			min	0	2	-.006	4	0	1	3.637e-6	15	NC	1	NC	1
197		4	max	.001	3	-.002	15	0	15	1.145e-4	1	NC	1	NC	1
198			min	-.001	2	-.009	4	0	1	5.328e-6	15	NC	1	NC	1
199		5	max	.002	3	-.003	15	0	15	1.509e-4	1	NC	1	NC	1
200			min	-.001	2	-.012	4	0	1	7.018e-6	15	8382.304	4	NC	1
201		6	max	.002	3	-.004	15	0	15	1.872e-4	1	NC	5	NC	1
202			min	-.002	2	-.015	4	0	1	8.709e-6	15	6802.371	4	NC	1
203		7	max	.003	3	-.004	15	0	15	2.236e-4	1	NC	5	NC	1
204			min	-.002	2	-.018	4	0	1	1.04e-5	15	5850.387	4	NC	1
205		8	max	.003	3	-.005	15	0	1	2.599e-4	1	NC	5	NC	1
206			min	-.002	2	-.02	4	0	12	1.209e-5	15	5263.42	4	NC	1
207		9	max	.003	3	-.005	15	0	1	2.963e-4	1	NC	5	NC	1
208			min	-.003	2	-.021	4	0	12	1.378e-5	15	4917.683	4	NC	1
209		10	max	.004	3	-.005	15	.001	1	3.327e-4	1	NC	5	NC	1
210			min	-.003	2	-.022	4	0	15	1.547e-5	15	4753.226	4	NC	1
211		11	max	.004	3	-.005	15	.002	1	3.69e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212			min	-.003	2	-.022	4	0	15	1.716e-5	15	4745.913	4	NC	1
213		12	max	.005	3	-.005	15	.002	1	4.054e-4	1	NC	5	NC	1
214			min	-.004	2	-.021	4	0	15	1.885e-5	15	4898.163	4	NC	1
215		13	max	.005	3	-.005	15	.003	1	4.417e-4	1	NC	5	NC	1
216			min	-.004	2	-.02	4	0	15	2.054e-5	15	5241.08	4	NC	1
217		14	max	.006	3	-.004	15	.004	1	4.781e-4	1	NC	5	NC	1
218			min	-.004	2	-.018	4	0	15	2.223e-5	15	5850.451	4	NC	1
219		15	max	.006	3	-.004	15	.006	1	5.145e-4	1	NC	3	NC	1
220			min	-.005	2	-.015	4	0	15	2.392e-5	15	6893.779	4	NC	1
221		16	max	.006	3	-.003	15	.007	1	5.508e-4	1	NC	1	NC	1
222			min	-.005	2	-.012	4	0	15	2.562e-5	15	8776.38	4	NC	1
223		17	max	.007	3	-.002	15	.009	1	5.872e-4	1	NC	1	NC	1
224			min	-.005	2	-.009	4	0	15	2.731e-5	15	NC	1	NC	1
225		18	max	.007	3	-.001	15	.011	1	6.235e-4	1	NC	1	NC	2
226			min	-.006	2	-.005	1	0	15	2.9e-5	15	NC	1	9284.565	1
227		19	max	.008	3	0	10	.013	1	6.599e-4	1	NC	1	NC	2
228			min	-.006	2	-.002	1	0	15	3.069e-5	15	NC	1	7746.103	1
229	M4	1	max	.003	1	.006	2	0	15	2.336e-4	1	NC	1	NC	3
230			min	0	3	-.008	3	-.013	1	1.09e-5	15	NC	1	1889.998	1
231		2	max	.003	1	.005	2	0	15	2.336e-4	1	NC	1	NC	3
232			min	0	3	-.007	3	-.012	1	1.09e-5	15	NC	1	2050.998	1
233		3	max	.002	1	.005	2	0	15	2.336e-4	1	NC	1	NC	3
234			min	0	3	-.007	3	-.011	1	1.09e-5	15	NC	1	2242.877	1
235		4	max	.002	1	.005	2	0	15	2.336e-4	1	NC	1	NC	3
236			min	0	3	-.007	3	-.01	1	1.09e-5	15	NC	1	2473.611	1
237		5	max	.002	1	.004	2	0	15	2.336e-4	1	NC	1	NC	3
238			min	0	3	-.006	3	-.009	1	1.09e-5	15	NC	1	2754.042	1
239		6	max	.002	1	.004	2	0	15	2.336e-4	1	NC	1	NC	3
240			min	0	3	-.006	3	-.008	1	1.09e-5	15	NC	1	3099.203	1
241		7	max	.002	1	.004	2	0	15	2.336e-4	1	NC	1	NC	3
242			min	0	3	-.005	3	-.007	1	1.09e-5	15	NC	1	3530.439	1
243		8	max	.002	1	.004	2	0	15	2.336e-4	1	NC	1	NC	3
244			min	0	3	-.005	3	-.006	1	1.09e-5	15	NC	1	4078.869	1
245		9	max	.001	1	.003	2	0	15	2.336e-4	1	NC	1	NC	2
246			min	0	3	-.004	3	-.005	1	1.09e-5	15	NC	1	4791.337	1
247		10	max	.001	1	.003	2	0	15	2.336e-4	1	NC	1	NC	2
248			min	0	3	-.004	3	-.004	1	1.09e-5	15	NC	1	5741.052	1
249		11	max	.001	1	.003	2	0	15	2.336e-4	1	NC	1	NC	2
250			min	0	3	-.004	3	-.004	1	1.09e-5	15	NC	1	7047.74	1
251		12	max	.001	1	.002	2	0	15	2.336e-4	1	NC	1	NC	2
252			min	0	3	-.003	3	-.003	1	1.09e-5	15	NC	1	8918.438	1
253		13	max	0	1	.002	2	0	15	2.336e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	-.002	1	1.09e-5	15	NC	1	NC	1
255		14	max	0	1	.002	2	0	15	2.336e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	-.002	1	1.09e-5	15	NC	1	NC	1
257		15	max	0	1	.001	2	0	15	2.336e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	-.001	1	1.09e-5	15	NC	1	NC	1
259		16	max	0	1	0	2	0	15	2.336e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	1.09e-5	15	NC	1	NC	1
261		17	max	0	1	0	2	0	15	2.336e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	1.09e-5	15	NC	1	NC	1
263		18	max	0	1	0	2	0	15	2.336e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	1.09e-5	15	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.336e-4	1	NC	1	NC	1
266			min	0	1	0	1	0	1	1.09e-5	15	NC	1	NC	1
267	M6	1	max	.024	1	.033	2	0	1	0	1	NC	3	NC	1
268			min	-.029	3	-.046	3	0	1	0	1	2317.09	2	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.023	1	.03	2	0	1	0	1	NC	3	NC	1
270		min	-.027	3	-.043	3	0	1	0	1	2554.046	2	NC	1
271	3	max	.021	1	.027	2	0	1	0	1	NC	3	NC	1
272		min	-.026	3	-.041	3	0	1	0	1	2842.305	2	NC	1
273	4	max	.02	1	.024	2	0	1	0	1	NC	3	NC	1
274		min	-.024	3	-.039	3	0	1	0	1	3197.199	2	NC	1
275	5	max	.019	1	.021	2	0	1	0	1	NC	3	NC	1
276		min	-.022	3	-.036	3	0	1	0	1	3640.453	2	NC	1
277	6	max	.017	1	.018	2	0	1	0	1	NC	3	NC	1
278		min	-.021	3	-.034	3	0	1	0	1	4203.695	2	NC	1
279	7	max	.016	1	.016	2	0	1	0	1	NC	1	NC	1
280		min	-.019	3	-.031	3	0	1	0	1	4934.451	2	NC	1
281	8	max	.015	1	.013	2	0	1	0	1	NC	1	NC	1
282		min	-.018	3	-.029	3	0	1	0	1	5906.891	2	NC	1
283	9	max	.013	1	.011	2	0	1	0	1	NC	1	NC	1
284		min	-.016	3	-.026	3	0	1	0	1	7242.254	2	NC	1
285	10	max	.012	1	.008	2	0	1	0	1	NC	1	NC	1
286		min	-.014	3	-.024	3	0	1	0	1	9150.438	2	NC	1
287	11	max	.011	1	.006	2	0	1	0	1	NC	1	NC	1
288		min	-.013	3	-.021	3	0	1	0	1	NC	1	NC	1
289	12	max	.009	1	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.011	3	-.019	3	0	1	0	1	NC	1	NC	1
291	13	max	.008	1	.003	2	0	1	0	1	NC	1	NC	1
292		min	-.01	3	-.016	3	0	1	0	1	NC	1	NC	1
293	14	max	.007	1	.002	2	0	1	0	1	NC	1	NC	1
294		min	-.008	3	-.013	3	0	1	0	1	NC	1	NC	1
295	15	max	.005	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.006	3	-.011	3	0	1	0	1	NC	1	NC	1
297	16	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
299	17	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	15	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.01	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.013	3	0	1	0	1	8575.994	4	NC	1
315	6	max	.007	3	-.004	15	0	1	0	1	NC	1	NC	1
316		min	-.006	2	-.016	3	0	1	0	1	6945.511	4	NC	1
317	7	max	.008	3	-.004	15	0	1	0	1	NC	2	NC	1
318		min	-.008	2	-.018	3	0	1	0	1	5963.699	4	NC	1
319	8	max	.009	3	-.005	15	0	1	0	1	NC	5	NC	1
320		min	-.009	2	-.02	4	0	1	0	1	5358.107	4	NC	1
321	9	max	.011	3	-.005	15	0	1	0	1	NC	5	NC	1
322		min	-.01	2	-.021	4	0	1	0	1	5000.504	4	NC	1
323	10	max	.012	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.011	2	-.022	4	0	1	0	1	4828.698	4	NC	1
325	11	max	.013	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.022	4	0	1	0	1	4817.415	4	NC	1
327		12	max	.015	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.014	2	-.021	4	0	1	0	1	4968.602	4	NC	1
329		13	max	.016	3	-.005	15	0	1	0	1	NC	5	NC	1
330			min	-.015	2	-.02	4	0	1	0	1	5313.433	4	NC	1
331		14	max	.017	3	-.004	15	0	1	0	1	NC	5	NC	1
332			min	-.017	2	-.018	4	0	1	0	1	5928.415	4	NC	1
333		15	max	.019	3	-.004	15	0	1	0	1	NC	1	NC	1
334			min	-.018	2	-.016	3	0	1	0	1	6982.951	4	NC	1
335		16	max	.02	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.019	2	-.014	3	0	1	0	1	8887.204	4	NC	1
337		17	max	.021	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.02	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.022	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.022	2	-.009	3	0	1	0	1	NC	1	NC	1
341		19	max	.024	3	0	10	0	1	0	1	NC	1	NC	1
342			min	-.023	2	-.006	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.022	2	0	1	0	1	NC	1	NC	1
344			min	0	3	-.025	3	0	1	0	1	NC	1	NC	1
345		2	max	.006	1	.021	2	0	1	0	1	NC	1	NC	1
346			min	0	3	-.023	3	0	1	0	1	NC	1	NC	1
347		3	max	.006	1	.02	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.022	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.018	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.02	3	0	1	0	1	NC	1	NC	1
351		5	max	.005	1	.017	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.019	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	1	.016	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.018	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.015	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	1	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.012	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	1	.011	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.012	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.006	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.008	1	.009	2	0	15	3.33e-4	1	NC	1	NC	2
382			min	-.009	3	-.015	3	-.014	1	1.553e-5	15	8686.98	2	5469.957	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	1	.007	2	0	15	3.161e-4	1	NC	1	NC	2
384		min	-.008	3	-.014	3	-.013	1	1.474e-5	15	NC	1	5961.094	1
385	3	max	.007	1	.006	2	0	15	2.992e-4	1	NC	1	NC	2
386		min	-.008	3	-.014	3	-.012	1	1.395e-5	15	NC	1	6545.426	1
387	4	max	.006	1	.005	2	0	15	2.823e-4	1	NC	1	NC	2
388		min	-.007	3	-.014	3	-.011	1	1.317e-5	15	NC	1	7247.383	1
389	5	max	.006	1	.003	2	0	15	2.653e-4	1	NC	1	NC	2
390		min	-.007	3	-.013	3	-.01	1	1.238e-5	15	NC	1	8100.205	1
391	6	max	.005	1	.002	2	0	15	2.484e-4	1	NC	1	NC	2
392		min	-.006	3	-.013	3	-.008	1	1.159e-5	15	NC	1	9150.058	1
393	7	max	.005	1	0	2	0	15	2.315e-4	1	NC	1	NC	1
394		min	-.006	3	-.012	3	-.007	1	1.08e-5	15	NC	1	NC	1
395	8	max	.005	1	0	2	0	15	2.146e-4	1	NC	1	NC	1
396		min	-.005	3	-.012	3	-.006	1	1.001e-5	15	NC	1	NC	1
397	9	max	.004	1	-.001	2	0	15	1.977e-4	1	NC	1	NC	1
398		min	-.005	3	-.011	3	-.005	1	9.226e-6	15	NC	1	NC	1
399	10	max	.004	1	-.002	15	0	15	1.808e-4	1	NC	1	NC	1
400		min	-.004	3	-.01	3	-.005	1	8.438e-6	15	NC	1	NC	1
401	11	max	.003	1	-.002	15	0	15	1.639e-4	1	NC	1	NC	1
402		min	-.004	3	-.01	3	-.004	1	7.65e-6	15	NC	1	NC	1
403	12	max	.003	1	-.002	15	0	15	1.47e-4	1	NC	1	NC	1
404		min	-.003	3	-.009	3	-.003	1	6.863e-6	15	NC	1	NC	1
405	13	max	.003	1	-.002	15	0	15	1.3e-4	1	NC	1	NC	1
406		min	-.003	3	-.008	3	-.002	1	6.075e-6	15	NC	1	NC	1
407	14	max	.002	1	-.002	15	0	15	1.131e-4	1	NC	1	NC	1
408		min	-.002	3	-.007	3	-.002	1	5.287e-6	15	NC	1	NC	1
409	15	max	.002	1	-.001	15	0	15	9.622e-5	1	NC	1	NC	1
410		min	-.002	3	-.006	4	-.001	1	4.499e-6	15	NC	1	NC	1
411	16	max	.001	1	-.001	15	0	15	7.93e-5	1	NC	1	NC	1
412		min	-.001	3	-.005	4	0	1	3.711e-6	15	NC	1	NC	1
413	17	max	0	1	0	15	0	15	6.239e-5	1	NC	1	NC	1
414		min	0	3	-.003	4	0	1	2.923e-6	15	NC	1	NC	1
415	18	max	0	1	0	15	0	15	4.548e-5	1	NC	1	NC	1
416		min	0	3	-.002	4	0	1	2.135e-6	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	2.856e-5	1	NC	1	NC	1
418		min	0	1	0	1	0	1	1.347e-6	15	NC	1	NC	1
419	M11	1	max	0	0	1	0	1	-2.561e-7	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-5.413e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-1.947e-6	15	NC	1	NC	1
422		min	0	2	-.003	4	0	15	-4.177e-5	1	NC	1	NC	1
423	3	max	0	3	-.001	15	0	1	-3.637e-6	15	NC	1	NC	1
424		min	0	2	-.006	4	0	15	-7.813e-5	1	NC	1	NC	1
425	4	max	.001	3	-.002	15	0	1	-5.328e-6	15	NC	1	NC	1
426		min	-.001	2	-.009	4	0	15	-1.145e-4	1	NC	1	NC	1
427	5	max	.002	3	-.003	15	0	1	-7.018e-6	15	NC	1	NC	1
428		min	-.001	2	-.012	4	0	15	-1.509e-4	1	8382.304	4	NC	1
429	6	max	.002	3	-.004	15	0	1	-8.709e-6	15	NC	5	NC	1
430		min	-.002	2	-.015	4	0	15	-1.872e-4	1	6802.371	4	NC	1
431	7	max	.003	3	-.004	15	0	1	-1.04e-5	15	NC	5	NC	1
432		min	-.002	2	-.018	4	0	15	-2.236e-4	1	5850.387	4	NC	1
433	8	max	.003	3	-.005	15	0	12	-1.209e-5	15	NC	5	NC	1
434		min	-.002	2	-.02	4	0	1	-2.599e-4	1	5263.42	4	NC	1
435	9	max	.003	3	-.005	15	0	12	-1.378e-5	15	NC	5	NC	1
436		min	-.003	2	-.021	4	0	1	-2.963e-4	1	4917.683	4	NC	1
437	10	max	.004	3	-.005	15	0	15	-1.547e-5	15	NC	5	NC	1
438		min	-.003	2	-.022	4	-.001	1	-3.327e-4	1	4753.226	4	NC	1
439	11	max	.004	3	-.005	15	0	15	-1.716e-5	15	NC	5	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440			min	-.003	2	-.022	4	-.002	1	-3.69e-4	1	4745.913	4	NC	1
441		12	max	.005	3	-.005	15	0	15	-1.885e-5	15	NC	5	NC	1
442			min	-.004	2	-.021	4	-.002	1	-4.054e-4	1	4898.163	4	NC	1
443		13	max	.005	3	-.005	15	0	15	-2.054e-5	15	NC	5	NC	1
444			min	-.004	2	-.02	4	-.003	1	-4.417e-4	1	5241.08	4	NC	1
445		14	max	.006	3	-.004	15	0	15	-2.223e-5	15	NC	5	NC	1
446			min	-.004	2	-.018	4	-.004	1	-4.781e-4	1	5850.451	4	NC	1
447		15	max	.006	3	-.004	15	0	15	-2.392e-5	15	NC	3	NC	1
448			min	-.005	2	-.015	4	-.006	1	-5.145e-4	1	6893.779	4	NC	1
449		16	max	.006	3	-.003	15	0	15	-2.562e-5	15	NC	1	NC	1
450			min	-.005	2	-.012	4	-.007	1	-5.508e-4	1	8776.38	4	NC	1
451		17	max	.007	3	-.002	15	0	15	-2.731e-5	15	NC	1	NC	1
452			min	-.005	2	-.009	4	-.009	1	-5.872e-4	1	NC	1	NC	1
453		18	max	.007	3	-.001	15	0	15	-2.9e-5	15	NC	1	NC	2
454			min	-.006	2	-.005	1	-.011	1	-6.235e-4	1	NC	1	9284.565	1
455		19	max	.008	3	0	10	0	15	-3.069e-5	15	NC	1	NC	2
456			min	-.006	2	-.002	1	-.013	1	-6.599e-4	1	NC	1	7746.103	1
457	M12	1	max	.003	1	.006	2	.013	1	-1.09e-5	15	NC	1	NC	3
458			min	0	3	-.008	3	0	15	-2.336e-4	1	NC	1	1889.998	1
459		2	max	.003	1	.005	2	.012	1	-1.09e-5	15	NC	1	NC	3
460			min	0	3	-.007	3	0	15	-2.336e-4	1	NC	1	2050.998	1
461		3	max	.002	1	.005	2	.011	1	-1.09e-5	15	NC	1	NC	3
462			min	0	3	-.007	3	0	15	-2.336e-4	1	NC	1	2242.877	1
463		4	max	.002	1	.005	2	.01	1	-1.09e-5	15	NC	1	NC	3
464			min	0	3	-.007	3	0	15	-2.336e-4	1	NC	1	2473.611	1
465		5	max	.002	1	.004	2	.009	1	-1.09e-5	15	NC	1	NC	3
466			min	0	3	-.006	3	0	15	-2.336e-4	1	NC	1	2754.042	1
467		6	max	.002	1	.004	2	.008	1	-1.09e-5	15	NC	1	NC	3
468			min	0	3	-.006	3	0	15	-2.336e-4	1	NC	1	3099.203	1
469		7	max	.002	1	.004	2	.007	1	-1.09e-5	15	NC	1	NC	3
470			min	0	3	-.005	3	0	15	-2.336e-4	1	NC	1	3530.439	1
471		8	max	.002	1	.004	2	.006	1	-1.09e-5	15	NC	1	NC	3
472			min	0	3	-.005	3	0	15	-2.336e-4	1	NC	1	4078.869	1
473		9	max	.001	1	.003	2	.005	1	-1.09e-5	15	NC	1	NC	2
474			min	0	3	-.004	3	0	15	-2.336e-4	1	NC	1	4791.337	1
475		10	max	.001	1	.003	2	.004	1	-1.09e-5	15	NC	1	NC	2
476			min	0	3	-.004	3	0	15	-2.336e-4	1	NC	1	5741.052	1
477		11	max	.001	1	.003	2	.004	1	-1.09e-5	15	NC	1	NC	2
478			min	0	3	-.004	3	0	15	-2.336e-4	1	NC	1	7047.74	1
479		12	max	.001	1	.002	2	.003	1	-1.09e-5	15	NC	1	NC	2
480			min	0	3	-.003	3	0	15	-2.336e-4	1	NC	1	8918.438	1
481		13	max	0	1	.002	2	.002	1	-1.09e-5	15	NC	1	NC	1
482			min	0	3	-.003	3	0	15	-2.336e-4	1	NC	1	NC	1
483		14	max	0	1	.002	2	.002	1	-1.09e-5	15	NC	1	NC	1
484			min	0	3	-.002	3	0	15	-2.336e-4	1	NC	1	NC	1
485		15	max	0	1	.001	2	.001	1	-1.09e-5	15	NC	1	NC	1
486			min	0	3	-.002	3	0	15	-2.336e-4	1	NC	1	NC	1
487		16	max	0	1	0	2	0	1	-1.09e-5	15	NC	1	NC	1
488			min	0	3	-.001	3	0	15	-2.336e-4	1	NC	1	NC	1
489		17	max	0	1	0	2	0	1	-1.09e-5	15	NC	1	NC	1
490			min	0	3	0	3	0	15	-2.336e-4	1	NC	1	NC	1
491		18	max	0	1	0	2	0	1	-1.09e-5	15	NC	1	NC	1
492			min	0	3	0	3	0	15	-2.336e-4	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-1.09e-5	15	NC	1	NC	1
494			min	0	1	0	1	0	1	-2.336e-4	1	NC	1	NC	1
495	M1	1	max	.01	3	.176	2	.001	1	1.363e-2	1	NC	1	NC	1
496			min	-.005	2	-.035	3	0	15	-2.325e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.01	3	.085	2	0	15	6.563e-3	1	NC	5	NC	1
498		min	-.005	2	-.016	3	-.01	1	-1.154e-2	3	1492.897	2	NC	1
499	3	max	.01	3	.015	3	0	15	-2.868e-7	10	NC	5	NC	2
500		min	-.005	2	-.012	2	-.014	1	-2.973e-4	1	720.457	2	8954.789	1
501	4	max	.01	3	.065	3	0	15	4.653e-3	2	NC	15	NC	2
502		min	-.005	2	-.121	2	-.013	1	-4.722e-3	3	456.103	2	9643.041	1
503	5	max	.009	3	.128	3	0	15	9.467e-3	1	9864.306	15	NC	1
504		min	-.005	2	-.235	2	-.009	1	-9.326e-3	3	329.798	2	NC	1
505	6	max	.009	3	.196	3	0	15	1.435e-2	1	7783.457	15	NC	1
506		min	-.005	2	-.345	2	-.004	1	-1.393e-2	3	260.094	2	NC	1
507	7	max	.009	3	.261	3	0	1	1.923e-2	1	6556.288	15	NC	1
508		min	-.005	2	-.443	2	0	3	-1.853e-2	3	218.905	2	NC	1
509	8	max	.009	3	.316	3	.001	1	2.411e-2	1	5830.806	15	NC	1
510		min	-.005	2	-.521	2	0	15	-2.314e-2	3	194.53	2	NC	1
511	9	max	.009	3	.351	3	0	15	2.667e-2	1	5451.695	15	NC	1
512		min	-.004	2	-.57	2	0	1	-2.346e-2	3	181.823	2	NC	1
513	10	max	.008	3	.364	3	0	1	2.867e-2	2	5335.936	15	NC	1
514		min	-.004	2	-.586	2	0	15	-2.091e-2	3	178.095	2	NC	1
515	11	max	.008	3	.356	3	0	1	3.068e-2	2	5451.455	15	NC	1
516		min	-.004	2	-.57	2	0	15	-1.836e-2	3	182.45	2	NC	1
517	12	max	.008	3	.326	3	0	15	2.955e-2	2	5830.307	15	NC	1
518		min	-.004	2	-.518	2	-.001	1	-1.558e-2	3	196.406	2	NC	1
519	13	max	.008	3	.277	3	0	15	2.372e-2	2	6555.428	15	NC	1
520		min	-.004	2	-.437	2	0	1	-1.246e-2	3	223.409	2	NC	1
521	14	max	.008	3	.216	3	.003	1	1.79e-2	2	7782.018	15	NC	1
522		min	-.004	2	-.335	2	0	15	-9.346e-3	3	269.64	2	NC	1
523	15	max	.007	3	.146	3	.008	1	1.207e-2	2	9861.841	15	NC	1
524		min	-.004	2	-.223	2	0	15	-6.227e-3	3	349.307	2	NC	1
525	16	max	.007	3	.074	3	.012	1	6.24e-3	2	NC	15	NC	1
526		min	-.004	2	-.11	2	0	15	-3.109e-3	3	496.91	2	NC	1
527	17	max	.007	3	.005	3	.013	1	8.497e-4	1	NC	5	NC	2
528		min	-.004	2	-.006	2	0	15	8.591e-6	12	804.887	1	9539.203	1
529	18	max	.007	3	.083	1	.009	1	9.861e-3	2	NC	5	NC	1
530		min	-.004	2	-.057	3	0	15	-3.607e-3	3	1696.958	1	NC	1
531	19	max	.007	3	.161	1	0	15	1.958e-2	2	NC	1	NC	1
532		min	-.004	2	-.115	3	-.001	1	-7.345e-3	3	NC	1	NC	1
533	M5	1	max	.03	.369	2	0	1	0	1	NC	1	NC	1
534		min	-.021	2	-.034	3	0	1	0	1	NC	1	NC	1
535	2	max	.03	3	.177	2	0	1	0	1	NC	5	NC	1
536		min	-.021	2	-.012	3	0	1	0	1	710.862	2	NC	1
537	3	max	.03	3	.047	3	0	1	0	1	NC	15	NC	1
538		min	-.021	2	-.039	2	0	1	0	1	333.836	2	NC	1
539	4	max	.029	3	.17	3	0	1	0	1	6861.773	15	NC	1
540		min	-.021	2	-.298	2	0	1	0	1	204.099	2	NC	1
541	5	max	.029	3	.339	3	0	1	0	1	4786.935	15	NC	1
542		min	-.02	2	-.579	2	0	1	0	1	143.414	2	NC	1
543	6	max	.028	3	.528	3	0	1	0	1	3676.468	15	NC	1
544		min	-.02	2	-.858	2	0	1	0	1	110.695	2	NC	1
545	7	max	.028	3	.713	3	0	1	0	1	3036.617	15	NC	1
546		min	-.019	2	-1.112	2	0	1	0	1	91.733	2	NC	1
547	8	max	.027	3	.868	3	0	1	0	1	2665.339	15	NC	1
548		min	-.019	2	-1.315	2	0	1	0	1	80.684	2	NC	1
549	9	max	.026	3	.968	3	0	1	0	1	2474.999	15	NC	1
550		min	-.019	2	-1.445	2	0	1	0	1	75.004	2	NC	1
551	10	max	.026	3	1.004	3	0	1	0	1	2417.612	15	NC	1
552		min	-.018	2	-1.488	2	0	1	0	1	73.344	2	NC	1
553	11	max	.025	3	.979	3	0	1	0	1	2475.1	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.018	2	-1.444	2	0	1	0	1	75.284	2	NC	1
555	12	max	.024	3	.894	3	0	1	0	1	2665.584	15	NC	1
556		min	-.018	2	-1.31	2	0	1	0	1	81.591	2	NC	1
557	13	max	.024	3	.757	3	0	1	0	1	3037.129	15	NC	1
558		min	-.017	2	-1.095	2	0	1	0	1	94.073	2	NC	1
559	14	max	.023	3	.585	3	0	1	0	1	3677.486	15	NC	1
560		min	-.017	2	-.83	2	0	1	0	1	115.976	2	NC	1
561	15	max	.022	3	.392	3	0	1	0	1	4788.972	15	NC	1
562		min	-.017	2	-.542	2	0	1	0	1	154.768	1	NC	1
563	16	max	.022	3	.197	3	0	1	0	1	6866.079	15	NC	1
564		min	-.017	2	-.262	2	0	1	0	1	227.006	1	NC	1
565	17	max	.021	3	.015	3	0	1	0	1	NC	15	NC	1
566		min	-.016	2	-.02	2	0	1	0	1	385.973	1	NC	1
567	18	max	.021	3	.173	1	0	1	0	1	NC	5	NC	1
568		min	-.016	2	-.14	3	0	1	0	1	847.118	1	NC	1
569	19	max	.021	3	.33	1	0	1	0	1	NC	1	NC	1
570		min	-.016	2	-.279	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.176	2	0	15	2.325e-2	3	NC	1	NC	1
572		min	-.005	2	-.035	3	-.001	1	-1.363e-2	1	NC	1	NC	1
573	2	max	.01	3	.085	2	.01	1	1.154e-2	3	NC	5	NC	1
574		min	-.005	2	-.016	3	0	15	-6.563e-3	1	1492.897	2	NC	1
575	3	max	.01	3	.015	3	.014	1	2.973e-4	1	NC	5	NC	2
576		min	-.005	2	-.012	2	0	15	2.868e-7	10	720.457	2	8954.789	1
577	4	max	.01	3	.065	3	.013	1	4.722e-3	3	NC	15	NC	2
578		min	-.005	2	-.121	2	0	15	-4.653e-3	2	456.103	2	9643.041	1
579	5	max	.009	3	.128	3	.009	1	9.326e-3	3	9864.306	15	NC	1
580		min	-.005	2	-.235	2	0	15	-9.467e-3	1	329.798	2	NC	1
581	6	max	.009	3	.196	3	.004	1	1.393e-2	3	7783.457	15	NC	1
582		min	-.005	2	-.345	2	0	15	-1.435e-2	1	260.094	2	NC	1
583	7	max	.009	3	.261	3	0	3	1.853e-2	3	6556.288	15	NC	1
584		min	-.005	2	-.443	2	0	1	-1.923e-2	1	218.905	2	NC	1
585	8	max	.009	3	.316	3	0	15	2.314e-2	3	5830.806	15	NC	1
586		min	-.005	2	-.521	2	-.001	1	-2.411e-2	1	194.53	2	NC	1
587	9	max	.009	3	.351	3	0	1	2.346e-2	3	5451.695	15	NC	1
588		min	-.004	2	-.57	2	0	15	-2.667e-2	1	181.823	2	NC	1
589	10	max	.008	3	.364	3	0	15	2.091e-2	3	5335.936	15	NC	1
590		min	-.004	2	-.586	2	0	1	-2.867e-2	2	178.095	2	NC	1
591	11	max	.008	3	.356	3	0	15	1.836e-2	3	5451.455	15	NC	1
592		min	-.004	2	-.57	2	0	1	-3.068e-2	2	182.45	2	NC	1
593	12	max	.008	3	.326	3	.001	1	1.558e-2	3	5830.307	15	NC	1
594		min	-.004	2	-.518	2	0	15	-2.955e-2	2	196.406	2	NC	1
595	13	max	.008	3	.277	3	0	1	1.246e-2	3	6555.428	15	NC	1
596		min	-.004	2	-.437	2	0	15	-2.372e-2	2	223.409	2	NC	1
597	14	max	.008	3	.216	3	0	15	9.346e-3	3	7782.018	15	NC	1
598		min	-.004	2	-.335	2	-.003	1	-1.79e-2	2	269.64	2	NC	1
599	15	max	.007	3	.146	3	0	15	6.227e-3	3	9861.841	15	NC	1
600		min	-.004	2	-.223	2	-.008	1	-1.207e-2	2	349.307	2	NC	1
601	16	max	.007	3	.074	3	0	15	3.109e-3	3	NC	15	NC	1
602		min	-.004	2	-.11	2	-.012	1	-6.24e-3	2	496.91	2	NC	1
603	17	max	.007	3	.005	3	0	15	-8.591e-6	12	NC	5	NC	2
604		min	-.004	2	-.006	2	-.013	1	-8.497e-4	1	804.887	1	9539.203	1
605	18	max	.007	3	.083	1	0	15	3.607e-3	3	NC	5	NC	1
606		min	-.004	2	-.057	3	-.009	1	-9.861e-3	2	1696.958	1	NC	1
607	19	max	.007	3	.161	1	.001	1	7.345e-3	3	NC	1	NC	1
608		min	-.004	2	-.115	3	0	15	-1.958e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

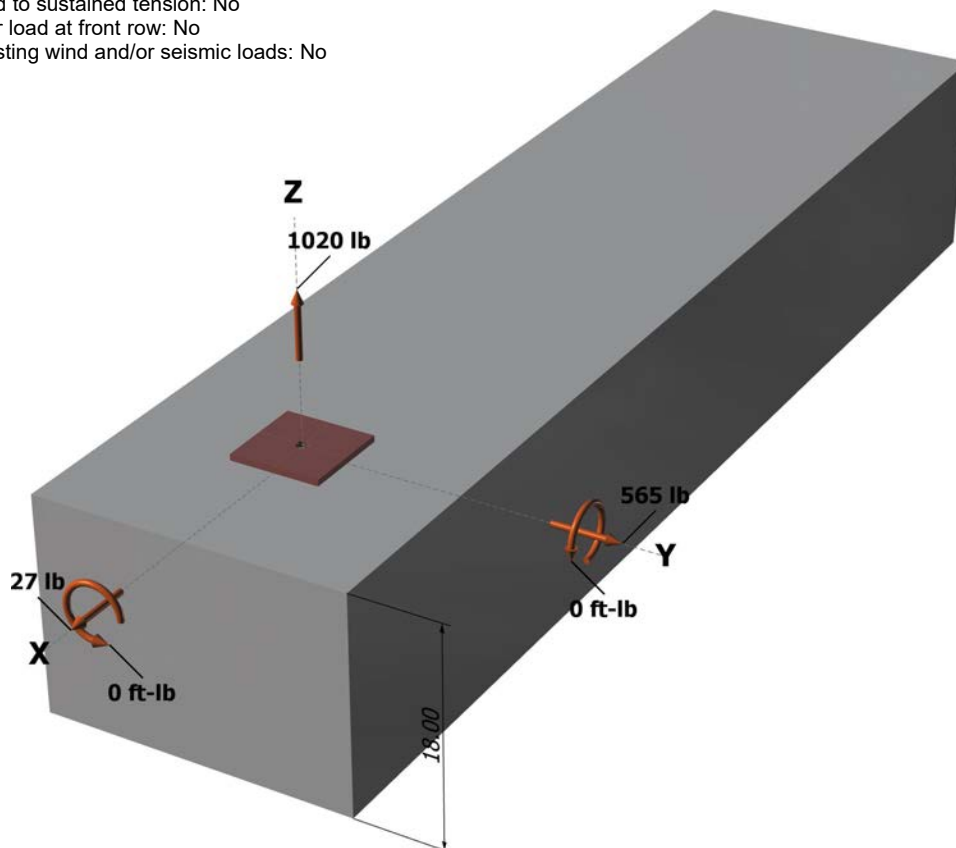
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 [www.strongtie.com](http://www.strongtie.com)



# Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

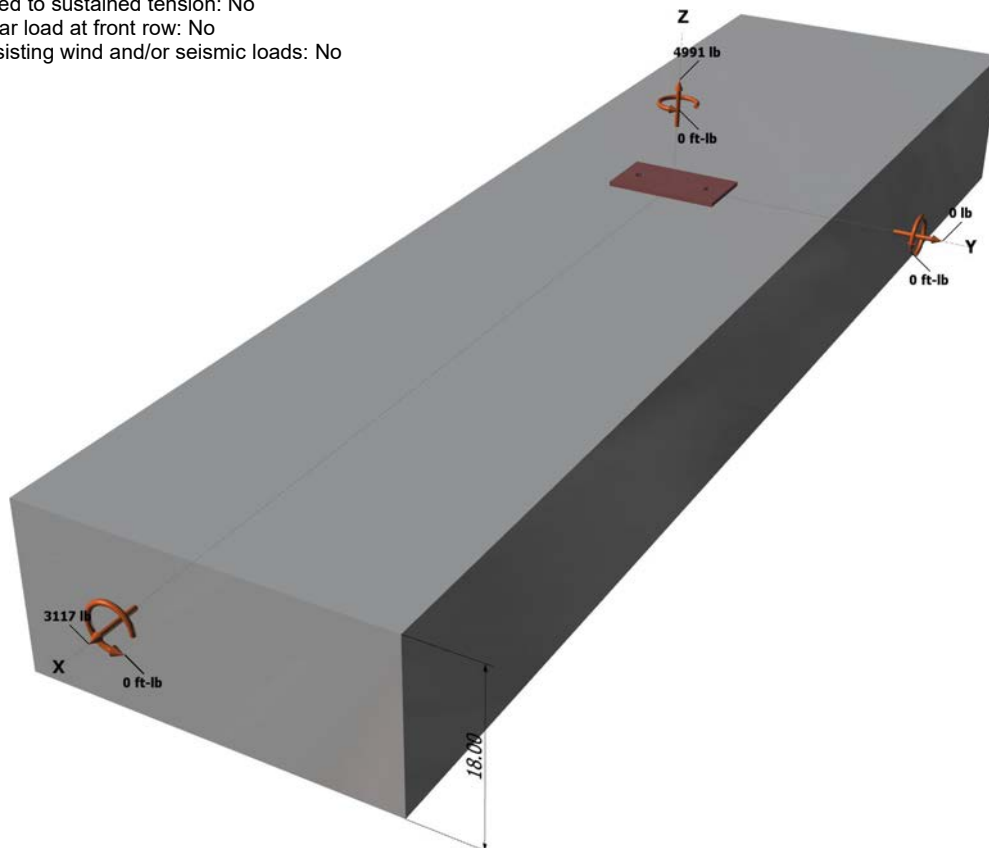
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

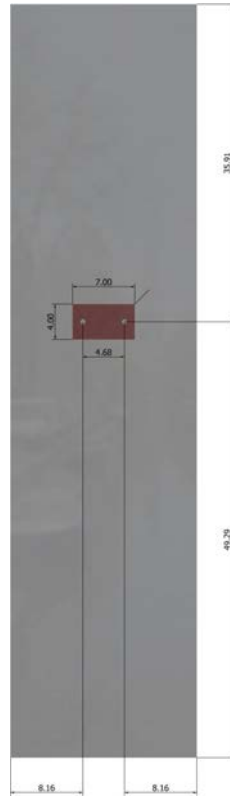
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 [www.strongtie.com](http://www.strongtie.com)





# Anchor Designer™ Software Version 2.4.6025.0

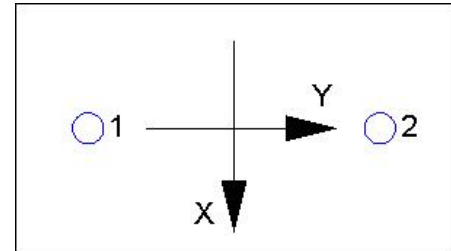
Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4991  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



**Anchor Designer™**  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.