

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	140 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 30.77 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

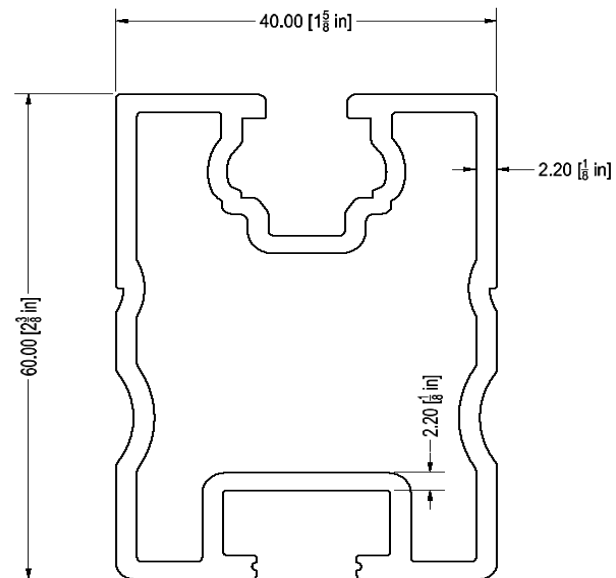
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

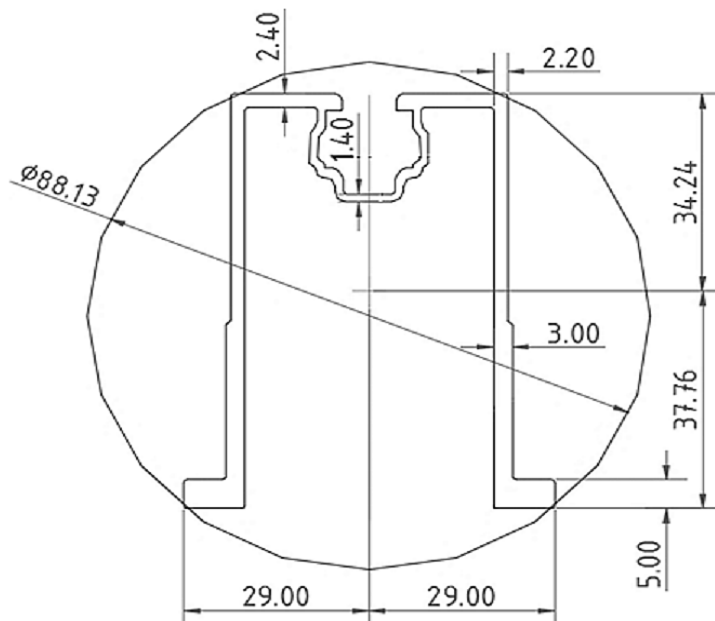
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	57 in
ΦF_{ty} STRONG-AXIS =	29.41 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.422 k-ft
M_z =	0.089 k-ft
$M_{y \text{ allowable}}$ =	1.251 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	44%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.65 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.516 k-ft
M_z =	0.000 k-ft
P_n =	0.276 k
$M_{y \text{ allowable}}$ =	1.455 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.031 k-ft
P_n =	0.173 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.458 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.762 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	13%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.184 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

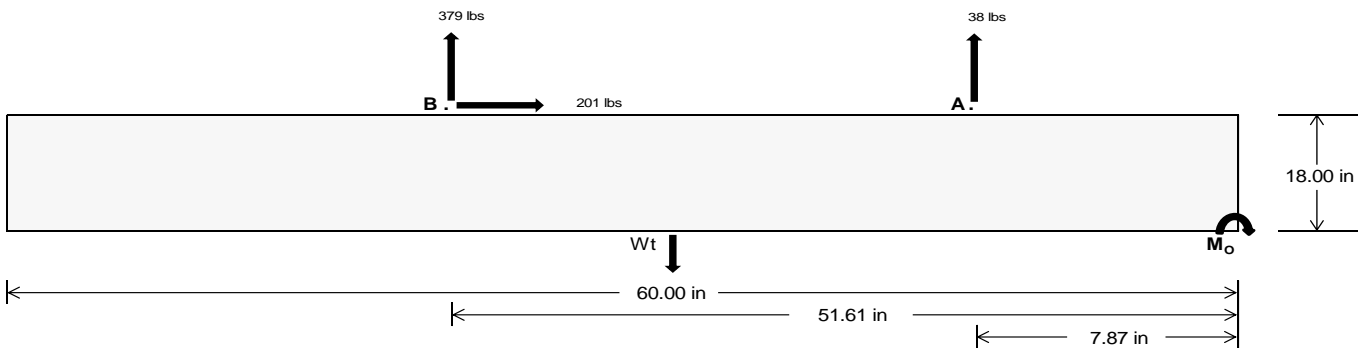
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>171.21</u>	<u>1645.31</u>	k
Compressive Load =	<u>1218.77</u>	<u>1132.45</u>	k
Lateral Load =	<u>25.85</u>	<u>869.34</u>	k
Moment (Weak Axis) =	<u>0.04</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23461.4$ in-lbs
Resisting Force Required = 782.05 lbs
S.F. = 1.67
Weight Required = 1303.41 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 200.53 lbs
Friction = 0.4
Weight Required = 501.34 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 200.53 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	418 lbs	418 lbs	418 lbs	418 lbs	426 lbs	426 lbs	426 lbs	426 lbs	598 lbs	598 lbs	598 lbs	598 lbs	-76 lbs	-76 lbs	-76 lbs	-76 lbs
F_B	295 lbs	295 lbs	295 lbs	295 lbs	463 lbs	463 lbs	463 lbs	463 lbs	544 lbs	544 lbs	544 lbs	544 lbs	-758 lbs	-758 lbs	-758 lbs	-758 lbs
F_V	38 lbs	38 lbs	38 lbs	38 lbs	359 lbs	359 lbs	359 lbs	359 lbs	294 lbs	294 lbs	294 lbs	294 lbs	-401 lbs	-401 lbs	-401 lbs	-401 lbs
P_{total}	2616 lbs	2707 lbs	2797 lbs	2888 lbs	2793 lbs	2883 lbs	2974 lbs	3064 lbs	3045 lbs	3136 lbs	3226 lbs	3317 lbs	308 lbs	362 lbs	417 lbs	471 lbs
M	295 lbs-ft	295 lbs-ft	295 lbs-ft	295 lbs-ft	489 lbs-ft	489 lbs-ft	489 lbs-ft	489 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	565 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft
e	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.18 ft	0.17 ft	0.16 ft	0.16 ft	0.19 ft	0.18 ft	0.18 ft	0.17 ft	2.02 ft	1.72 ft	1.49 ft	1.32 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	258.6 psf	256.7 psf	255.0 psf	253.4 psf	252.1 psf	250.5 psf	249.1 psf	247.7 psf	270.4 psf	268.0 psf	265.8 psf	263.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	339.4 psf	333.8 psf	328.8 psf	324.1 psf	386.2 psf	378.5 psf	371.5 psf	365.1 psf	425.5 psf	416.1 psf	407.4 psf	399.5 psf	244.7 psf	168.4 psf	144.0 psf	133.2 psf

Maximum Bearing Pressure = 426 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

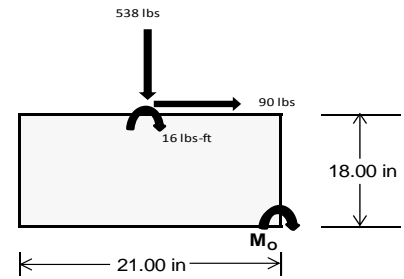
Overturning Check

$M_o = 319.5 \text{ ft-lbs}$
 Resisting Force Required = 365.11 lbs
 S.F. = 1.67
 Weight Required = 608.52 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	118 lbs	84 lbs	60 lbs	247 lbs	538 lbs	203 lbs	76 lbs	-21 lbs	21 lbs
F_v	14 lbs	120 lbs	15 lbs	10 lbs	90 lbs	11 lbs	15 lbs	119 lbs	15 lbs
P_{total}	2474 lbs	2441 lbs	2416 lbs	2490 lbs	2781 lbs	2446 lbs	765 lbs	668 lbs	710 lbs
M	41 lbs-ft	200 lbs-ft	43 lbs-ft	29 lbs-ft	151 lbs-ft	33 lbs-ft	41 lbs-ft	200 lbs-ft	43 lbs-ft
e	0.02 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.30 ft	0.06 ft
$L/6$	0.29 ft	1.59 ft	1.71 ft	1.73 ft	1.64 ft	1.72 ft	1.64 ft	1.15 ft	1.63 ft
f_{min}	266.8 sqft	200.4 sqft	259.4 sqft	273.1 sqft	258.6 sqft	266.7 sqft	71.4 sqft	-2.1 sqft	64.4 sqft
f_{max}	298.7 psf	357.4 psf	293.0 psf	296.0 psf	377.0 psf	292.4 psf	103.5 psf	154.8 psf	97.9 psf



Maximum Bearing Pressure = 377 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

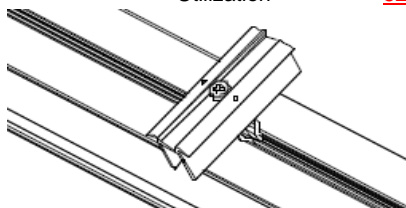
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

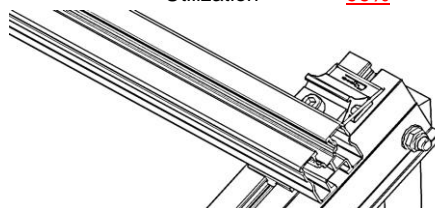
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.635 k
Allowable Uplift =	1.214 k
Utilization =	<u>52%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.074 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

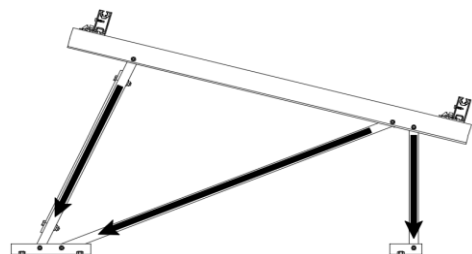
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.938 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

Diagonal Strut

Maximum Axial Load =	0.458 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.116 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.184 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.065 in
	<u>0.065 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$148.425$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 57.00 \text{ in}$$

$$J = 0.255$$

$$154.13$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.3$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.251 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.24 \\
 &22.039 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.24 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	240.39	1	.031	2	.223	1	0	10	0	4	0	15
30			min	-352.49	3	-.028	3	-.469	5	0	4	0	3	0	6
31		16	max	240.507	1	-.004	2	.223	1	0	10	0	4	0	15
32			min	-352.402	3	-.055	3	-.575	5	0	4	0	3	0	6
33		17	max	240.623	1	-.022	15	.223	1	0	10	0	1	0	15
34			min	-352.315	3	-.09	4	-.68	5	0	4	0	3	0	6
35		18	max	240.74	1	-.033	15	.223	1	0	10	0	1	0	15
36			min	-352.228	3	-.136	4	-.786	5	0	4	0	3	0	6
37		19	max	240.856	1	-.044	15	.223	1	0	10	0	1	0	15
38			min	-352.14	3	-.181	4	-.891	5	0	4	0	3	0	6
39	M3	1	max	127.625	2	1.776	6	-.011	10	0	5	0	1	0	6
40			min	-126.265	3	.417	15	-1.355	4	0	1	0	10	0	15
41		2	max	127.556	2	1.599	6	-.011	10	0	5	0	1	0	2
42			min	-126.316	3	.375	15	-1.222	4	0	1	0	10	0	15
43		3	max	127.488	2	1.422	6	-.011	10	0	5	0	1	0	2
44			min	-126.368	3	.333	15	-1.088	4	0	1	0	5	0	3
45		4	max	127.419	2	1.244	6	-.011	10	0	5	0	1	0	15
46			min	-126.419	3	.292	15	-.954	4	0	1	0	5	0	4
47		5	max	127.351	2	1.067	6	-.011	10	0	5	0	1	0	15
48			min	-126.471	3	.25	15	-.821	4	0	1	0	5	0	4
49		6	max	127.282	2	.89	6	-.011	10	0	5	0	1	0	15
50			min	-126.522	3	.208	15	-.687	4	0	1	0	5	0	4
51		7	max	127.213	2	.713	6	-.011	10	0	5	0	1	0	15
52			min	-126.573	3	.167	15	-.554	4	0	1	0	5	0	4
53		8	max	127.145	2	.536	6	-.011	10	0	5	0	1	0	15
54			min	-126.625	3	.125	15	-.42	4	0	1	0	5	-.001	4
55		9	max	127.076	2	.358	6	-.011	10	0	5	0	1	0	15
56			min	-126.676	3	.083	15	-.286	4	0	1	0	5	-.001	4
57		10	max	127.008	2	.181	6	-.011	10	0	5	0	1	0	15
58			min	-126.728	3	.042	15	-.232	1	0	1	0	5	-.001	4
59		11	max	126.939	2	.027	2	.031	5	0	5	0	1	0	15
60			min	-126.779	3	-.021	3	-.232	1	0	1	0	5	-.001	4
61		12	max	126.87	2	-.042	15	.165	5	0	5	0	1	0	15
62			min	-126.831	3	-.173	4	-.232	1	0	1	0	5	-.001	4
63		13	max	126.802	2	-.083	15	.299	5	0	5	0	1	0	15
64			min	-126.882	3	-.35	4	-.232	1	0	1	0	5	-.001	4
65		14	max	126.733	2	-.125	15	.432	5	0	5	0	1	0	15
66			min	-126.934	3	-.528	4	-.232	1	0	1	0	5	-.001	4
67		15	max	126.665	2	-.167	15	.566	5	0	5	0	1	0	15
68			min	-126.985	3	-.705	4	-.232	1	0	1	0	5	0	4
69		16	max	126.596	2	-.208	15	.7	5	0	5	0	10	0	15
70			min	-127.037	3	-.882	4	-.232	1	0	1	0	4	0	4
71		17	max	126.527	2	-.25	15	.833	5	0	5	0	10	0	15
72			min	-127.088	3	-1.059	4	-.232	1	0	1	0	4	0	4
73		18	max	126.459	2	-.292	15	.967	5	0	5	0	10	0	15
74			min	-127.139	3	-1.236	4	-.232	1	0	1	0	4	0	4
75		19	max	126.39	2	-.333	15	1.1	5	0	5	0	5	0	1
76			min	-127.191	3	-1.414	4	-.232	1	0	1	0	1	0	1
77	M4	1	max	332.03	1	0	1	-.036	10	0	1	0	5	0	1
78			min	-31.334	3	0	1	-18.755	4	0	1	0	2	0	1
79		2	max	332.094	1	0	1	-.036	10	0	1	0	12	0	1
80			min	-31.285	3	0	1	-18.812	4	0	1	-.002	4	0	1
81		3	max	332.159	1	0	1	-.036	10	0	1	0	10	0	1
82			min	-31.237	3	0	1	-18.868	4	0	1	-.003	4	0	1
83		4	max	332.224	1	0	1	-.036	10	0	1	0	10	0	1
84			min	-31.188	3	0	1	-18.924	4	0	1	-.005	4	0	1
85		5	max	332.288	1	0	1	-.036	10	0	1	0	10	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-31.14	3	0	1	-18.98	4	0	1	-.007	4	0	1
87		6	max	332.353	1	0	1	-.036	10	0	1	0	10	0	1
88			min	-31.091	3	0	1	-19.036	4	0	1	-.008	4	0	1
89		7	max	332.418	1	0	1	-.036	10	0	1	0	10	0	1
90			min	-31.043	3	0	1	-19.092	4	0	1	-.01	4	0	1
91		8	max	332.482	1	0	1	-.036	10	0	1	0	10	0	1
92			min	-30.994	3	0	1	-19.148	4	0	1	-.012	4	0	1
93		9	max	332.547	1	0	1	-.036	10	0	1	0	10	0	1
94			min	-30.946	3	0	1	-19.204	4	0	1	-.014	4	0	1
95		10	max	332.612	1	0	1	-.036	10	0	1	0	10	0	1
96			min	-30.897	3	0	1	-19.26	4	0	1	-.015	4	0	1
97		11	max	332.677	1	0	1	-.036	10	0	1	0	10	0	1
98			min	-30.849	3	0	1	-19.316	4	0	1	-.017	4	0	1
99		12	max	332.741	1	0	1	-.036	10	0	1	0	10	0	1
100			min	-30.8	3	0	1	-19.372	4	0	1	-.019	4	0	1
101		13	max	332.806	1	0	1	-.036	10	0	1	0	10	0	1
102			min	-30.751	3	0	1	-19.428	4	0	1	-.02	4	0	1
103		14	max	332.871	1	0	1	-.036	10	0	1	0	10	0	1
104			min	-30.703	3	0	1	-19.484	4	0	1	-.022	4	0	1
105		15	max	332.935	1	0	1	-.036	10	0	1	0	10	0	1
106			min	-30.654	3	0	1	-19.541	4	0	1	-.024	4	0	1
107		16	max	333	1	0	1	-.036	10	0	1	0	10	0	1
108			min	-30.606	3	0	1	-19.597	4	0	1	-.026	4	0	1
109		17	max	333.065	1	0	1	-.036	10	0	1	0	10	0	1
110			min	-30.557	3	0	1	-19.653	4	0	1	-.027	4	0	1
111		18	max	333.13	1	0	1	-.036	10	0	1	0	10	0	1
112			min	-30.509	3	0	1	-19.709	4	0	1	-.029	4	0	1
113		19	max	333.194	1	0	1	-.036	10	0	1	0	10	0	1
114			min	-30.46	3	0	1	-19.765	4	0	1	-.031	4	0	1
115	M6	1	max	759.914	1	.629	6	1.013	4	0	3	0	3	0	1
116			min	-1115.805	3	.142	15	-.225	3	0	5	0	2	0	1
117		2	max	760.03	1	.584	6	.908	4	0	3	0	4	0	15
118			min	-1115.718	3	.131	15	-.225	3	0	5	0	2	0	6
119		3	max	760.146	1	.538	6	.802	4	0	3	0	4	0	15
120			min	-1115.631	3	.12	15	-.225	3	0	5	0	2	0	6
121		4	max	760.263	1	.492	6	.697	4	0	3	0	4	0	15
122			min	-1115.543	3	.11	15	-.225	3	0	5	0	2	0	6
123		5	max	760.379	1	.451	2	.592	4	0	3	0	4	0	15
124			min	-1115.456	3	.099	15	-.225	3	0	5	0	2	0	6
125		6	max	760.496	1	.416	2	.486	4	0	3	0	4	0	15
126			min	-1115.369	3	.088	15	-.225	3	0	5	0	3	0	6
127		7	max	760.612	1	.38	2	.381	4	0	3	0	4	0	15
128			min	-1115.282	3	.077	15	-.225	3	0	5	0	3	0	6
129		8	max	760.728	1	.345	2	.275	4	0	3	0	4	0	15
130			min	-1115.194	3	.067	15	-.225	3	0	5	0	3	0	6
131		9	max	760.845	1	.309	2	.17	4	0	3	0	4	0	15
132			min	-1115.107	3	.051	12	-.225	3	0	5	0	3	0	2
133		10	max	760.961	1	.273	2	.068	14	0	3	0	4	0	15
134			min	-1115.02	3	.034	12	-.225	3	0	5	0	3	0	2
135		11	max	761.078	1	.238	2	.058	1	0	3	0	4	0	15
136			min	-1114.932	3	.014	3	-.225	3	0	5	0	3	0	2
137		12	max	761.194	1	.202	2	.058	1	0	3	0	4	0	15
138			min	-1114.845	3	-.013	3	-.225	3	0	5	0	3	0	2
139		13	max	761.31	1	.167	2	.058	1	0	3	0	4	0	15
140			min	-1114.758	3	-.039	3	-.273	5	0	5	0	3	0	2
141		14	max	761.427	1	.131	2	.058	1	0	3	0	4	0	15
142			min	-1114.67	3	-.066	3	-.378	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	761.543	1	.095	2	.058	1	0	3	0	4	0	15
144		min	-1114.583	3	-.093	3	-.484	5	0	5	0	3	0	2
145	16	max	761.66	1	.06	2	.058	1	0	3	0	4	0	12
146		min	-1114.496	3	-.119	3	-.589	5	0	5	0	3	0	2
147	17	max	761.776	1	.024	2	.058	1	0	3	0	4	0	12
148		min	-1114.409	3	-.146	3	-.695	5	0	5	0	3	0	2
149	18	max	761.893	1	-.011	2	.058	1	0	3	0	4	0	12
150		min	-1114.321	3	-.173	3	-.8	5	0	5	0	3	0	2
151	19	max	762.009	1	-.047	2	.058	1	0	3	0	4	0	12
152		min	-1114.234	3	-.199	3	-.906	5	0	5	0	3	0	2
153	M7	1	max	457.514	2	1.79	.016	3	0	1	0	4	0	2
154		min	-368.333	3	.425	15	-1.359	4	0	3	0	3	0	12
155	2	max	457.445	2	1.613	4	.016	3	0	1	0	4	0	2
156		min	-368.384	3	.384	15	-1.225	4	0	3	0	3	0	3
157	3	max	457.377	2	1.436	4	.016	3	0	1	0	1	0	2
158		min	-368.436	3	.342	15	-1.092	4	0	3	0	3	0	3
159	4	max	457.308	2	1.258	4	.016	3	0	1	0	1	0	2
160		min	-368.487	3	.301	15	-.958	4	0	3	0	3	0	3
161	5	max	457.24	2	1.081	4	.016	3	0	1	0	1	0	15
162		min	-368.539	3	.259	15	-.825	4	0	3	0	5	0	3
163	6	max	457.171	2	.904	4	.016	3	0	1	0	1	0	15
164		min	-368.59	3	.217	15	-.691	4	0	3	0	5	0	6
165	7	max	457.102	2	.727	4	.016	3	0	1	0	1	0	15
166		min	-368.641	3	.176	15	-.557	4	0	3	0	5	0	6
167	8	max	457.034	2	.55	4	.016	3	0	1	0	1	0	15
168		min	-368.693	3	.134	15	-.424	4	0	3	0	5	0	6
169	9	max	456.965	2	.372	4	.016	3	0	1	0	1	0	15
170		min	-368.744	3	.092	15	-.29	4	0	3	0	5	-.001	6
171	10	max	456.897	2	.216	2	.016	3	0	1	0	1	0	15
172		min	-368.796	3	.025	12	-.156	4	0	3	0	5	-.001	6
173	11	max	456.828	2	.078	2	.016	3	0	1	0	1	0	15
174		min	-368.847	3	-.074	3	-.023	4	0	3	0	5	-.001	6
175	12	max	456.759	2	-.033	15	.112	5	0	1	0	1	0	15
176		min	-368.899	3	-.177	3	-.008	1	0	3	0	5	-.001	6
177	13	max	456.691	2	-.074	15	.245	5	0	1	0	1	0	15
178		min	-368.95	3	-.337	6	-.008	1	0	3	0	5	-.001	6
179	14	max	456.622	2	-.116	15	.379	5	0	1	0	1	0	15
180		min	-369.002	3	-.514	6	-.008	1	0	3	0	5	-.001	6
181	15	max	456.554	2	-.158	15	.512	5	0	1	0	1	0	15
182		min	-369.053	3	-.691	6	-.008	1	0	3	0	5	0	6
183	16	max	456.485	2	-.199	15	.646	5	0	1	0	1	0	15
184		min	-369.105	3	-.868	6	-.008	1	0	3	0	5	0	6
185	17	max	456.416	2	-.241	15	.78	5	0	1	0	1	0	15
186		min	-369.156	3	-1.046	6	-.008	1	0	3	0	5	0	6
187	18	max	456.348	2	-.283	15	.913	5	0	1	0	1	0	15
188		min	-369.207	3	-1.223	6	-.008	1	0	3	0	5	0	6
189	19	max	456.279	2	-.324	15	1.047	5	0	1	0	1	0	1
190		min	-369.259	3	-1.4	6	-.008	1	0	3	0	3	0	1
191	M8	1	max	936.354	1	0	.309	1	0	1	0	4	0	1
192		min	-132.574	3	0	1	-19.043	4	0	1	0	1	0	1
193	2	max	936.419	1	0	1	.309	1	0	1	0	1	0	1
194		min	-132.526	3	0	1	-19.099	4	0	1	-.002	4	0	1
195	3	max	936.484	1	0	1	.309	1	0	1	0	1	0	1
196		min	-132.477	3	0	1	-19.155	4	0	1	-.003	4	0	1
197	4	max	936.548	1	0	1	.309	1	0	1	0	1	0	1
198		min	-132.429	3	0	1	-19.212	4	0	1	-.005	4	0	1
199	5	max	936.613	1	0	1	.309	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-132.38	3	0	1	-19.268	4	0	1	-.007	4	0	1
201		6	max	936.678	1	0	1	.309	1	0	1	0	1	0	1
202			min	-132.332	3	0	1	-19.324	4	0	1	-.009	4	0	1
203		7	max	936.742	1	0	1	.309	1	0	1	0	1	0	1
204			min	-132.283	3	0	1	-19.38	4	0	1	-.01	4	0	1
205		8	max	936.807	1	0	1	.309	1	0	1	0	1	0	1
206			min	-132.235	3	0	1	-19.436	4	0	1	-.012	4	0	1
207		9	max	936.872	1	0	1	.309	1	0	1	0	1	0	1
208			min	-132.186	3	0	1	-19.492	4	0	1	-.014	4	0	1
209		10	max	936.937	1	0	1	.309	1	0	1	0	1	0	1
210			min	-132.137	3	0	1	-19.548	4	0	1	-.016	4	0	1
211		11	max	937.001	1	0	1	.309	1	0	1	0	1	0	1
212			min	-132.089	3	0	1	-19.604	4	0	1	-.017	4	0	1
213		12	max	937.066	1	0	1	.309	1	0	1	0	1	0	1
214			min	-132.04	3	0	1	-19.66	4	0	1	-.019	4	0	1
215		13	max	937.131	1	0	1	.309	1	0	1	0	1	0	1
216			min	-131.992	3	0	1	-19.716	4	0	1	-.021	4	0	1
217		14	max	937.195	1	0	1	.309	1	0	1	0	1	0	1
218			min	-131.943	3	0	1	-19.772	4	0	1	-.023	4	0	1
219		15	max	937.26	1	0	1	.309	1	0	1	0	1	0	1
220			min	-131.895	3	0	1	-19.828	4	0	1	-.024	4	0	1
221		16	max	937.325	1	0	1	.309	1	0	1	0	1	0	1
222			min	-131.846	3	0	1	-19.884	4	0	1	-.026	4	0	1
223		17	max	937.389	1	0	1	.309	1	0	1	0	1	0	1
224			min	-131.798	3	0	1	-19.941	4	0	1	-.028	4	0	1
225		18	max	937.454	1	0	1	.309	1	0	1	0	1	0	1
226			min	-131.749	3	0	1	-19.997	4	0	1	-.03	4	0	1
227		19	max	937.519	1	0	1	.309	1	0	1	0	1	0	1
228			min	-131.701	3	0	1	-20.053	4	0	1	-.031	4	0	1
229	M10	1	max	240.753	1	.67	4	1.146	5	0	1	0	1	0	1
230			min	-312.19	3	.169	15	-.113	1	-.001	5	0	3	0	1
231		2	max	240.869	1	.624	4	1.04	5	0	1	0	4	0	15
232			min	-312.103	3	.159	15	-.113	1	-.001	5	0	3	0	4
233		3	max	240.986	1	.579	4	.935	5	0	1	0	4	0	15
234			min	-312.015	3	.148	15	-.113	1	-.001	5	0	3	0	4
235		4	max	241.102	1	.533	4	.829	5	0	1	0	4	0	15
236			min	-311.928	3	.137	15	-.113	1	-.001	5	0	3	0	4
237		5	max	241.219	1	.487	4	.724	5	0	1	0	4	0	15
238			min	-311.841	3	.126	15	-.113	1	-.001	5	0	3	0	4
239		6	max	241.335	1	.442	4	.618	5	0	1	0	4	0	15
240			min	-311.753	3	.116	15	-.113	1	-.001	5	0	3	0	4
241		7	max	241.451	1	.396	4	.513	5	0	1	0	4	0	15
242			min	-311.666	3	.105	15	-.113	1	-.001	5	0	3	0	4
243		8	max	241.568	1	.35	4	.407	5	0	1	0	4	0	15
244			min	-311.579	3	.094	15	-.113	1	-.001	5	0	3	0	4
245		9	max	241.684	1	.305	4	.302	5	0	1	0	4	0	15
246			min	-311.491	3	.083	15	-.113	1	-.001	5	0	3	0	4
247		10	max	241.801	1	.259	4	.196	5	0	1	0	4	0	15
248			min	-311.404	3	.073	15	-.113	1	-.001	5	0	3	0	4
249		11	max	241.917	1	.213	4	.091	5	0	1	0	5	0	15
250			min	-311.317	3	.062	15	-.113	1	-.001	5	0	3	0	4
251		12	max	242.033	1	.168	4	-.01	10	0	1	0	5	0	15
252			min	-311.23	3	.051	15	-.113	1	-.001	5	0	3	0	4
253		13	max	242.15	1	.122	4	-.01	10	0	1	0	5	0	15
254			min	-311.142	3	.034	12	-.135	4	-.001	5	0	3	0	4
255		14	max	242.266	1	.076	4	-.01	10	0	1	0	5	0	15
256			min	-311.055	3	.016	12	-.24	4	-.001	5	0	3	0	4



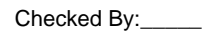
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	242.383	1	.031	2	-.01	10	0	1	0	5	0	15
258		min	-310.968	3	-.004	9	-.346	4	-.001	5	0	3	0	4
259	16	max	242.499	1	.012	5	-.01	10	0	1	0	5	0	15
260		min	-310.88	3	-.033	9	-.451	4	-.001	5	0	3	0	4
261	17	max	242.615	1	-.002	15	-.01	10	0	1	0	5	0	15
262		min	-310.793	3	-.063	1	-.557	4	-.001	5	0	3	0	4
263	18	max	242.732	1	-.013	15	-.01	10	0	1	0	5	0	15
264		min	-310.706	3	-.107	6	-.662	4	-.001	5	0	3	0	4
265	19	max	242.848	1	-.024	15	-.01	10	0	1	0	5	0	15
266		min	-310.618	3	-.153	6	-.768	4	-.001	5	0	1	0	4
267	M11	1	max	127.162	2	1.77	.253	1	0	4	0	5	0	6
268		min	-126.926	3	.412	15	-1.244	5	0	10	0	1	0	15
269	2	max	127.093	2	1.593	6	.253	1	0	4	0	5	0	2
270		min	-126.977	3	.371	15	-1.111	5	0	10	0	1	0	12
271	3	max	127.025	2	1.415	6	.253	1	0	4	0	5	0	2
272		min	-127.029	3	.329	15	-.977	5	0	10	0	1	0	3
273	4	max	126.956	2	1.238	6	.253	1	0	4	0	3	0	15
274		min	-127.08	3	.287	15	-.843	5	0	10	0	1	0	4
275	5	max	126.888	2	1.061	6	.253	1	0	4	0	3	0	15
276		min	-127.132	3	.246	15	-.71	5	0	10	0	1	0	4
277	6	max	126.819	2	.884	6	.253	1	0	4	0	3	0	15
278		min	-127.183	3	.204	15	-.576	5	0	10	0	1	0	4
279	7	max	126.75	2	.707	6	.253	1	0	4	0	3	0	15
280		min	-127.235	3	.162	15	-.443	5	0	10	0	1	0	4
281	8	max	126.682	2	.529	6	.253	1	0	4	0	3	0	15
282		min	-127.286	3	.121	15	-.309	5	0	10	0	4	-.001	4
283	9	max	126.613	2	.352	6	.253	1	0	4	0	3	0	15
284		min	-127.338	3	.079	15	-.175	5	0	10	0	4	-.001	4
285	10	max	126.545	2	.175	6	.253	1	0	4	0	3	0	15
286		min	-127.389	3	.037	15	-.042	5	0	10	0	4	-.001	4
287	11	max	126.476	2	.027	2	.253	1	0	4	0	3	0	15
288		min	-127.441	3	-.035	3	-.031	3	0	10	0	4	-.001	4
289	12	max	126.407	2	-.046	15	.281	4	0	4	0	3	0	15
290		min	-127.492	3	-.18	4	-.031	3	0	10	0	4	-.001	4
291	13	max	126.339	2	-.088	15	.415	4	0	4	0	3	0	15
292		min	-127.543	3	-.357	4	-.031	3	0	10	0	4	-.001	4
293	14	max	126.27	2	-.129	15	.548	4	0	4	0	3	0	15
294		min	-127.595	3	-.534	4	-.031	3	0	10	0	4	-.001	4
295	15	max	126.202	2	-.171	15	.682	4	0	4	0	3	0	15
296		min	-127.646	3	-.711	4	-.031	3	0	10	0	5	0	4
297	16	max	126.133	2	-.213	15	.815	4	0	4	0	3	0	15
298		min	-127.698	3	-.889	4	-.031	3	0	10	0	10	0	4
299	17	max	126.064	2	-.254	15	.949	4	0	4	0	4	0	15
300		min	-127.749	3	-1.066	4	-.031	3	0	10	0	10	0	4
301	18	max	125.996	2	-.296	15	1.083	4	0	4	0	4	0	15
302		min	-127.801	3	-1.243	4	-.031	3	0	10	0	10	0	4
303	19	max	125.927	2	-.338	15	1.216	4	0	4	0	4	0	1
304		min	-127.852	3	-1.42	4	-.031	3	0	10	0	10	0	1
305	M12	1	max	332.01	1	0	1.305	1	0	1	0	4	0	1
306		min	-30.844	3	0	1	-17.466	5	0	1	0	3	0	1
307	2	max	332.075	1	0	1	1.305	1	0	1	0	1	0	1
308		min	-30.795	3	0	1	-17.522	5	0	1	-.002	5	0	1
309	3	max	332.14	1	0	1	1.305	1	0	1	0	1	0	1
310		min	-30.747	3	0	1	-17.578	5	0	1	-.003	5	0	1
311	4	max	332.205	1	0	1	1.305	1	0	1	0	1	0	1
312		min	-30.698	3	0	1	-17.634	5	0	1	-.005	5	0	1
313	5	max	332.269	1	0	1	1.305	1	0	1	0	1	0	1



RISA-3D Version 13.0.0 \...\...\PVMMini 60 Cell 1V 25° 140mph 30psf 4.75ft 7-10Pa Page 27



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	59.161	3	2.363	9	-1.197	10	0	3	-.001	10	.153	2
372			min	-5.905	10	-21.813	2	-26.904	1	0	1	-.029	1	-.096	3
373		16	max	84.161	2	76.357	2	-1.207	10	0	1	-.002	10	.157	2
374			min	-19.418	3	-119.62	3	-27.11	1	0	5	-.035	1	-.09	3
375		17	max	84.279	2	76.104	2	-1.207	10	0	1	-.002	10	.141	2
376			min	-19.329	3	-119.809	3	-27.11	1	0	5	-.041	1	-.064	3
377		18	max	-3.924	12	325.915	2	-1.243	10	0	3	-.002	10	.071	2
378			min	-87.149	1	-148.434	3	-31.969	4	0	2	-.047	1	-.032	3
379		19	max	-3.865	12	325.662	2	-1.243	10	0	3	-.002	10	0	2
380			min	-87.031	1	-148.623	3	-31.727	4	0	2	-.053	1	0	3
381	M5	1	max	204.059	1	1084.175	3	0	2	0	1	.033	4	0	3
382			min	2.946	12	-780.237	1	-57.508	3	0	5	0	10	0	2
383		2	max	204.177	1	1083.985	3	0	2	0	1	.028	4	.169	1
384			min	3.005	12	-780.49	1	-57.508	3	0	5	-.005	3	-.235	3
385		3	max	165.08	3	5.691	9	6.355	3	0	3	.023	4	.335	1
386			min	-24.006	10	-67.78	2	-18.116	4	0	4	-.017	3	-.465	3
387		4	max	165.169	3	5.48	9	6.355	3	0	3	.02	4	.341	1
388			min	-23.907	10	-68.033	2	-17.874	4	0	4	-.015	3	-.451	3
389		5	max	165.257	3	5.269	9	6.355	3	0	3	.016	4	.353	2
390			min	-23.809	10	-68.286	2	-17.632	4	0	4	-.014	3	-.437	3
391		6	max	165.346	3	5.058	9	6.355	3	0	3	.012	4	.368	2
392			min	-23.711	10	-68.539	2	-17.39	4	0	4	-.013	3	-.424	3
393		7	max	165.434	3	4.848	9	6.355	3	0	3	.008	4	.383	2
394			min	-23.612	10	-68.792	2	-17.148	4	0	4	-.011	3	-.41	3
395		8	max	165.523	3	4.637	9	6.355	3	0	3	.004	4	.398	2
396			min	-23.514	10	-69.046	2	-16.906	4	0	4	-.01	3	-.396	3
397		9	max	165.611	3	4.426	9	6.355	3	0	3	0	4	.413	2
398			min	-23.416	10	-69.299	2	-16.664	4	0	4	-.008	3	-.382	3
399		10	max	165.7	3	4.215	9	6.355	3	0	3	0	2	.428	2
400			min	-23.317	10	-69.552	2	-16.422	4	0	4	-.007	3	-.369	3
401		11	max	165.788	3	4.004	9	6.355	3	0	3	0	2	.443	2
402			min	-23.219	10	-69.805	2	-16.18	4	0	4	-.006	4	-.355	3
403		12	max	165.877	3	3.793	9	6.355	3	0	3	0	2	.458	2
404			min	-23.121	10	-70.058	2	-15.938	4	0	4	-.01	4	-.341	3
405		13	max	165.965	3	3.582	9	6.355	3	0	3	0	2	.473	2
406			min	-23.022	10	-70.311	2	-15.696	4	0	4	-.013	4	-.327	3
407		14	max	166.054	3	3.371	9	6.355	3	0	3	0	2	.488	2
408			min	-22.924	10	-70.564	2	-15.454	4	0	4	-.017	4	-.313	3
409		15	max	166.142	3	3.16	9	6.355	3	0	3	0	2	.504	2
410			min	-22.826	10	-70.817	2	-15.212	4	0	4	-.02	4	-.299	3
411		16	max	275.768	2	288.106	2	6.325	3	0	3	0	3	.516	2
412			min	-63.476	3	-353.899	3	-13.929	4	0	4	-.023	4	-.282	3
413		17	max	275.886	2	287.853	2	6.325	3	0	3	.002	3	.454	2
414			min	-63.388	3	-354.089	3	-13.687	4	0	4	-.026	4	-.205	3
415		18	max	-5.163	12	1053.91	2	5.817	3	0	4	.003	3	.228	2
416			min	-204.21	1	-474.926	3	-32.04	5	0	1	-.033	4	-.103	3
417		19	max	-5.104	12	1053.657	2	5.817	3	0	4	.005	3	0	3
418			min	-204.092	1	-475.116	3	-31.798	5	0	1	-.04	4	0	2
419	M9	1	max	86.809	1	333.335	3	133.926	4	0	3	0	15	0	2
420			min	1.178	15	-240.58	1	1.206	10	0	1	-.053	1	0	3
421		2	max	86.928	1	333.145	3	134.168	4	0	3	.027	5	.052	1
422			min	1.214	15	-240.834	1	1.206	10	0	1	-.047	1	-.073	3
423		3	max	57.901	3	4.874	9	26.361	1	0	1	.053	5	.104	1
424			min	-6.714	10	-18.749	2	-23.466	5	0	5	-.04	1	-.143	3
425		4	max	57.989	3	4.663	9	26.361	1	0	1	.048	5	.105	1
426			min	-6.616	10	-19.002	2	-23.224	5	0	5	-.034	1	-.14	3
427		5	max	58.078	3	4.452	9	26.361	1	0	1	.043	5	.109	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-6.517	10	-19.255	2	-22.982	5	0	5	-.029	1	-.136	3
429	6	max	58.166	3	4.241	9	26.361	1	0	1	.038	5	.113	2
430		min	-6.419	10	-19.509	2	-22.74	5	0	5	-.023	1	-.132	3
431	7	max	58.255	3	4.031	9	26.361	1	0	1	.033	5	.117	2
432		min	-6.321	10	-19.762	2	-22.498	5	0	5	-.017	1	-.128	3
433	8	max	58.343	3	3.82	9	26.361	1	0	1	.029	5	.122	2
434		min	-6.222	10	-20.015	2	-22.256	5	0	5	-.011	1	-.124	3
435	9	max	58.432	3	3.609	9	26.361	1	0	1	.024	5	.126	2
436		min	-6.124	10	-20.268	2	-22.014	5	0	5	-.006	1	-.12	3
437	10	max	58.52	3	3.398	9	26.361	1	0	1	.019	4	.13	2
438		min	-6.026	10	-20.521	2	-21.772	5	0	5	0	1	-.116	3
439	11	max	58.609	3	3.187	9	26.361	1	0	1	.016	4	.135	2
440		min	-5.927	10	-20.774	2	-21.53	5	0	5	0	10	-.112	3
441	12	max	58.697	3	2.976	9	26.361	1	0	1	.012	4	.139	2
442		min	-5.829	10	-21.027	2	-21.288	5	0	5	0	10	-.108	3
443	13	max	58.786	3	2.765	9	26.361	1	0	1	.017	1	.144	2
444		min	-5.731	10	-21.28	2	-21.046	5	0	5	0	10	-.104	3
445	14	max	58.874	3	2.554	9	26.361	1	0	1	.023	1	.149	2
446		min	-5.632	10	-21.533	2	-20.804	5	0	5	0	15	-.1	3
447	15	max	58.963	3	2.343	9	26.361	1	0	1	.029	1	.153	2
448		min	-5.534	10	-21.786	2	-20.562	5	0	5	-.004	5	-.096	3
449	16	max	84.317	2	76.022	2	26.589	1	0	10	.035	1	.157	2
450		min	-20.018	3	-120.058	3	-19.161	5	0	4	-.007	5	-.091	3
451	17	max	84.435	2	75.769	2	26.589	1	0	10	.04	1	.141	2
452		min	-19.929	3	-120.248	3	-18.919	5	0	4	-.011	5	-.064	3
453	18	max	5.941	5	325.915	2	27.896	1	0	2	.046	1	.071	2
454		min	-86.911	1	-148.427	3	-36.125	5	0	3	-.019	5	-.032	3
455	19	max	5.996	5	325.662	2	27.896	1	0	2	.053	1	0	2
456		min	-86.793	1	-148.617	3	-35.883	5	0	3	-.027	5	0	3
457	M13	1	max	133.927	4	240.341	1	-1.178	15	0	.053	1	0	1
458		min	1.206	10	-333.36	3	-86.803	1	0	3	0	15	0	3
459	2	max	128.744	4	170.405	1	-.416	15	0	2	.012	1	.15	3
460		min	1.206	10	-236.094	3	-65.808	1	0	3	0	10	-.108	1
461	3	max	123.56	4	100.469	1	.418	5	0	2	.007	3	.249	3
462		min	1.206	10	-138.828	3	-44.813	1	0	3	-.017	1	-.18	1
463	4	max	118.376	4	30.532	1	1.598	5	0	2	.004	3	.297	3
464		min	1.206	10	-41.561	3	-23.817	1	0	3	-.035	1	-.214	1
465	5	max	113.193	4	55.705	3	2.777	5	0	2	.002	3	.293	3
466		min	1.206	10	-39.404	1	-3.407	3	0	3	-.042	1	-.212	1
467	6	max	108.009	4	152.972	3	18.174	1	0	2	.003	5	.238	3
468		min	1.206	10	-109.34	1	-2.297	3	0	3	-.038	1	-.173	1
469	7	max	102.825	4	250.238	3	39.169	1	0	2	.005	5	.132	3
470		min	1.206	10	-179.277	1	-1.188	3	0	3	-.023	1	-.097	1
471	8	max	97.642	4	347.504	3	60.164	1	0	2	.009	4	.016	1
472		min	1.206	10	-249.213	1	-.078	3	0	3	0	3	-.026	3
473	9	max	92.458	4	444.771	3	81.16	1	0	2	.041	1	.166	1
474		min	1.206	10	-319.149	1	.901	12	0	3	0	3	-.235	3
475	10	max	87.274	4	542.037	3	102.155	1	0	2	.089	1	.353	1
476		min	1.206	10	-389.086	1	1.641	12	0	3	-.007	3	-.496	3
477	11	max	61.163	4	319.149	1	4.343	5	0	3	.04	1	.166	1
478		min	1.206	10	-444.771	3	-80.921	1	0	2	-.014	5	-.235	3
479	12	max	55.979	4	249.213	1	5.522	5	0	3	.004	2	.016	1
480		min	1.206	10	-347.504	3	-59.926	1	0	2	-.011	5	-.026	3
481	13	max	50.796	4	179.277	1	6.702	5	0	3	-.001	10	.132	3
482		min	1.206	10	-250.238	3	-38.93	1	0	2	-.023	1	-.097	1
483	14	max	45.612	4	109.34	1	7.882	5	0	3	-.003	15	.238	3
484		min	1.206	10	-152.972	3	-17.935	1	0	2	-.038	1	-.173	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	40.428	4	39.404	1	10.033	4	0	3	0	5	.293	3
486			min	1.206	10	-55.705	3	-1.035	10	0	2	-.042	1	-.212	1
487		16	max	35.245	4	41.561	3	24.056	1	0	3	.005	5	.297	3
488			min	1.206	10	-30.532	1	1.071	10	0	2	-.035	1	-.214	1
489		17	max	30.061	4	138.828	3	45.051	1	0	3	.011	5	.249	3
490			min	1.206	10	-100.469	1	3.178	10	0	2	-.017	1	-.18	1
491		18	max	27.081	1	236.094	3	66.047	1	0	3	.021	4	.15	3
492			min	1.206	10	-170.405	1	4.619	12	0	2	0	10	-.108	1
493		19	max	27.081	1	333.36	3	87.042	1	0	3	.053	1	0	1
494			min	1.206	10	-240.342	1	5.359	12	0	2	.002	10	0	3
495	M16	1	max	35.873	5	325.781	2	5.996	5	0	3	.053	1	0	2
496			min	-27.838	1	-148.637	3	-86.799	1	0	2	-.027	5	0	3
497		2	max	30.69	5	230.985	2	7.175	5	0	3	.012	1	.067	3
498			min	-27.838	1	-105.723	3	-65.804	1	0	2	-.023	5	-.147	2
499		3	max	25.506	5	136.189	2	8.355	5	0	3	0	12	.112	3
500			min	-27.838	1	-62.809	3	-44.809	1	0	2	-.023	4	-.244	2
501		4	max	20.322	5	41.393	2	9.534	5	0	3	-.002	12	.133	3
502			min	-27.838	1	-19.894	3	-23.813	1	0	2	-.035	1	-.291	2
503		5	max	15.139	5	23.02	3	10.714	5	0	3	-.002	12	.133	3
504			min	-27.838	1	-53.403	2	-2.818	1	0	2	-.042	1	-.287	2
505		6	max	9.955	5	65.934	3	18.178	1	0	3	-.002	15	.109	3
506			min	-27.838	1	-148.199	2	-1.18	3	0	2	-.038	1	-.234	2
507		7	max	4.771	5	108.849	3	39.173	1	0	3	.003	5	.063	3
508			min	-27.838	1	-242.994	2	-.07	3	0	2	-.023	1	-.131	2
509		8	max	1.544	3	151.763	3	60.168	1	0	3	.011	4	.022	2
510			min	-27.838	1	-337.79	2	.832	12	0	2	-.005	3	-.006	3
511		9	max	1.544	3	194.677	3	81.164	1	0	3	.041	1	.225	2
512			min	-27.838	1	-432.586	2	1.571	12	0	2	-.004	3	-.097	3
513		10	max	21.029	5	-9.089	15	102.159	1	0	14	.089	1	.479	2
514			min	-27.838	1	-527.382	2	-4.067	3	0	2	-.003	3	-.211	3
515		11	max	15.846	5	432.586	2	3.754	5	0	2	.04	1	.225	2
516			min	-27.758	1	-194.677	3	-80.925	1	0	3	-.011	5	-.097	3
517		12	max	10.662	5	337.79	2	4.933	5	0	2	.004	2	.022	2
518			min	-27.758	1	-151.763	3	-59.93	1	0	3	-.009	5	-.006	3
519		13	max	5.478	5	242.994	2	6.113	5	0	2	-.001	10	.063	3
520			min	-27.758	1	-108.848	3	-38.935	1	0	3	-.023	1	-.131	2
521		14	max	.295	5	148.199	2	7.292	5	0	2	-.001	12	.109	3
522			min	-27.758	1	-65.934	3	-17.939	1	0	3	-.038	1	-.234	2
523		15	max	-1.243	10	53.403	2	9.421	4	0	2	.002	5	.133	3
524			min	-27.758	1	-23.02	3	-1.029	10	0	3	-.042	1	-.287	2
525		16	max	-1.243	10	19.894	3	24.051	1	0	2	.006	5	.133	3
526			min	-27.758	1	-41.393	2	1.077	10	0	3	-.035	1	-.291	2
527		17	max	-1.243	10	62.809	3	45.047	1	0	2	.012	5	.112	3
528			min	-27.758	1	-136.189	2	2.385	12	0	3	-.017	1	-.244	2
529		18	max	-1.243	10	105.723	3	66.042	1	0	2	.021	4	.067	3
530			min	-27.758	1	-230.985	2	3.125	12	0	3	0	10	-.147	2
531		19	max	-1.243	10	148.637	3	87.038	1	0	2	.053	1	0	2
532			min	-31.757	4	-325.781	2	3.865	12	0	3	.002	10	0	3
533	M15	1	max	0	1	.978	3	.096	3	0	1	0	1	0	1
534			min	-75.572	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.87	3	.096	3	0	1	0	1	0	1
536			min	-75.637	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.761	3	.096	3	0	1	0	1	0	1
538			min	-75.702	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.652	3	.096	3	0	1	0	1	0	1
540			min	-75.768	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.544	3	.096	3	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-75.833	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.435	3	.096	3	0	1	0	1	0	1
544			min	-75.898	3	0	1	0	1	0	3	0	3	-.001	3
545		7	max	0	1	.326	3	.096	3	0	1	0	3	0	1
546			min	-75.963	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.217	3	.096	3	0	1	0	3	0	1
548			min	-76.028	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.109	3	.096	3	0	1	0	3	0	1
550			min	-76.094	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.096	3	0	1	0	3	0	1
552			min	-76.159	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.096	3	0	1	0	3	0	1
554			min	-76.224	3	-.109	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.096	3	0	1	0	3	0	1
556			min	-76.289	3	-.217	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.096	3	0	1	0	3	0	1
558			min	-76.354	3	-.326	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.096	3	0	1	0	3	0	1
560			min	-76.42	3	-.435	3	0	1	0	3	0	1	-.001	3
561		15	max	0	1	0	1	.096	3	0	1	0	3	0	1
562			min	-76.485	3	-.544	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.096	3	0	1	0	3	0	1
564			min	-76.55	3	-.652	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.096	3	0	1	0	3	0	1
566			min	-76.615	3	-.761	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.096	3	0	1	0	3	0	1
568			min	-76.68	3	-.87	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.096	3	0	1	0	3	0	1
570			min	-76.745	3	-.978	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.341	4	.275	4	0	3	0	3	0	1
572			min	-183.376	4	0	2	-.039	3	0	1	0	4	0	1
573		2	max	0	2	2.081	4	.249	4	0	3	0	3	0	2
574			min	-183.388	4	0	2	-.039	3	0	1	0	4	0	4
575		3	max	0	2	1.821	4	.222	4	0	3	0	3	0	2
576			min	-183.4	4	0	2	-.039	3	0	1	0	4	-.001	4
577		4	max	0	2	1.56	4	.195	4	0	3	0	3	0	2
578			min	-183.412	4	0	2	-.039	3	0	1	0	1	-.002	4
579		5	max	0	2	1.3	4	.168	4	0	3	0	3	0	2
580			min	-183.424	4	0	2	-.039	3	0	1	0	1	-.002	4
581		6	max	0	2	1.04	4	.141	4	0	3	0	3	0	2
582			min	-183.436	4	0	2	-.039	3	0	1	0	1	-.003	4
583		7	max	0	2	.78	4	.114	4	0	3	0	5	0	2
584			min	-183.448	4	0	2	-.039	3	0	1	0	1	-.003	4
585		8	max	0	2	.52	4	.087	4	0	3	0	5	0	2
586			min	-183.46	4	0	2	-.039	3	0	1	0	1	-.003	4
587		9	max	0	2	.26	4	.061	4	0	3	0	5	0	2
588			min	-183.472	4	0	2	-.039	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.035	1	0	3	0	5	0	2
590			min	-183.484	4	0	1	-.039	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.035	1	0	3	0	5	0	2
592			min	-183.496	4	-.26	4	-.039	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.035	1	0	3	0	5	0	2
594			min	-183.508	4	-.52	4	-.039	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.035	1	0	3	0	5	0	2
596			min	-183.52	4	-.78	4	-.051	5	0	1	0	3	-.003	4
597		14	max	0	2	0	2	.035	1	0	3	0	5	0	2
598			min	-183.532	4	-1.04	4	-.077	5	0	1	0	3	-.003	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	2	0	2	.035	1	0	3	0	4	0	2
600		min	-183.544	4	-1.3	4	-.104	5	0	1	0	3	-.002	4
601	16	max	0	2	0	2	.035	1	0	3	0	4	0	2
602		min	-183.556	4	-1.56	4	-.131	5	0	1	0	3	-.002	4
603	17	max	.051	11	0	2	.035	1	0	3	0	1	0	2
604		min	-183.568	4	-1.821	4	-.158	5	0	1	0	3	-.001	4
605	18	max	.123	11	0	2	.035	1	0	3	0	1	0	2
606		min	-183.58	4	-2.081	4	-.185	5	0	1	0	3	0	4
607	19	max	.196	11	0	2	.035	1	0	3	0	1	0	1
608		min	-183.592	4	-2.341	4	-.212	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.008	2	.005	1	1.09e-3	5	NC	3	NC	2
2			min	-.003	3	-.007	3	-.011	5	-4.131e-4	1	4693.72	2	7261.958	1
3		2	max	.002	1	.007	2	.005	1	1.111e-3	5	NC	3	NC	2
4			min	-.003	3	-.007	3	-.01	5	-3.955e-4	1	5117.842	2	7841.324	1
5		3	max	.002	1	.006	2	.004	1	1.131e-3	5	NC	1	NC	2
6			min	-.003	3	-.007	3	-.01	5	-3.779e-4	1	5621.511	2	8524.922	1
7		4	max	.002	1	.006	2	.004	1	1.152e-3	5	NC	1	NC	2
8			min	-.003	3	-.007	3	-.01	5	-3.603e-4	1	6223.938	2	9337.92	1
9		5	max	.002	1	.005	2	.004	1	1.173e-3	5	NC	1	NC	1
10			min	-.002	3	-.006	3	-.009	5	-3.427e-4	1	6950.743	2	NC	1
11		6	max	.002	1	.005	2	.003	1	1.193e-3	5	NC	1	NC	1
12			min	-.002	3	-.006	3	-.009	5	-3.25e-4	1	7836.66	2	NC	1
13		7	max	.001	1	.004	2	.003	1	1.214e-3	5	NC	1	NC	1
14			min	-.002	3	-.006	3	-.009	5	-3.074e-4	1	8929.702	2	NC	1
15		8	max	.001	1	.004	2	.002	1	1.235e-3	5	NC	1	NC	1
16			min	-.002	3	-.005	3	-.008	5	-2.898e-4	1	NC	1	NC	1
17		9	max	.001	1	.003	2	.002	1	1.256e-3	5	NC	1	NC	1
18			min	-.002	3	-.005	3	-.008	5	-2.722e-4	1	NC	1	NC	1
19		10	max	.001	1	.003	2	.002	1	1.276e-3	5	NC	1	NC	1
20		min	-.002	3	-.005	3	-.007	5	-2.546e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	.002	1	1.297e-3	5	NC	1	NC	1
22			min	-.001	3	-.004	3	-.006	5	-2.369e-4	1	NC	1	NC	1
23		12	max	0	1	.002	2	.001	1	1.318e-3	5	NC	1	NC	1
24			min	-.001	3	-.004	3	-.006	5	-2.193e-4	1	NC	1	NC	1
25		13	max	0	1	.001	2	0	1	1.338e-3	5	NC	1	NC	1
26			min	-.001	3	-.003	3	-.005	5	-2.017e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	0	1	1.359e-3	5	NC	1	NC	1
28			min	0	3	-.003	3	-.004	5	-1.841e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	0	1	1.38e-3	5	NC	1	NC	1
30			min	0	3	-.002	3	-.003	5	-1.665e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	0	1	1.4e-3	5	NC	1	NC	1
32			min	0	3	-.002	3	-.003	5	-1.489e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	1.421e-3	5	NC	1	NC	1
34			min	0	3	-.001	3	-.002	5	-1.312e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	1.442e-3	5	NC	1	NC	1
36			min	0	3	0	3	0	5	-1.136e-4	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	1.462e-3	5	NC	1	NC	1
38			min	0	1	0	1	0	1	-9.599e-5	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	4.469e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-6.803e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.004	5	5.673e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	1	-6.854e-4	5	NC	1	NC	1



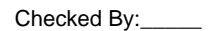
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	6.877e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.905e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.011	5	8.081e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.956e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	9.285e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-7.007e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.018	4	1.049e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-7.058e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.021	4	1.169e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-7.109e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.025	4	1.29e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-7.16e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.028	4	1.41e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	-7.211e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.031	4	1.53e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-7.262e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.034	4	1.651e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-7.313e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.037	4	1.771e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-7.364e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.04	4	1.892e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-7.415e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.043	4	2.012e-4	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	10	-7.466e-4	5	NC	1	NC	1
67		15	max	.001	3	.005	2	.046	4	2.132e-4	1	NC	1	NC	1
68			min	-.001	2	-.007	3	0	10	-7.517e-4	5	NC	1	NC	1
69		16	max	.001	3	.005	2	.049	4	2.253e-4	1	NC	1	NC	1
70			min	-.001	2	-.007	3	0	10	-7.568e-4	5	8466.723	2	NC	1
71		17	max	.001	3	.006	2	.052	4	2.373e-4	1	NC	1	NC	1
72			min	-.001	2	-.007	3	0	10	-7.619e-4	5	7230.115	2	NC	1
73		18	max	.001	3	.007	2	.054	4	2.494e-4	1	NC	3	NC	1
74			min	-.001	2	-.007	3	0	10	-7.67e-4	5	6273.957	2	NC	1
75		19	max	.001	3	.008	2	.057	4	2.614e-4	1	NC	3	NC	1
76			min	-.001	2	-.007	3	0	10	-7.721e-4	5	5526.807	2	NC	1
77	M4	1	max	.002	1	.009	2	0	10	3.412e-3	5	NC	1	NC	2
78			min	0	3	-.007	3	-.06	4	-3.366e-4	1	NC	1	320.083	4
79		2	max	.001	1	.008	2	0	10	3.412e-3	5	NC	1	NC	2
80			min	0	3	-.007	3	-.055	4	-3.366e-4	1	NC	1	348.908	4
81		3	max	.001	1	.008	2	0	10	3.412e-3	5	NC	1	NC	1
82			min	0	3	-.007	3	-.05	4	-3.366e-4	1	NC	1	383.213	4
83		4	max	.001	1	.007	2	0	10	3.412e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.046	4	-3.366e-4	1	NC	1	424.441	4
85		5	max	.001	1	.007	2	0	10	3.412e-3	5	NC	1	NC	1
86			min	0	3	-.006	3	-.041	4	-3.366e-4	1	NC	1	474.559	4
87		6	max	.001	1	.006	2	0	10	3.412e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.036	4	-3.366e-4	1	NC	1	536.3	4
89		7	max	.001	1	.006	2	0	10	3.412e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.031	4	-3.366e-4	1	NC	1	613.559	4
91		8	max	0	1	.005	2	0	10	3.412e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.027	4	-3.366e-4	1	NC	1	712.035	4
93		9	max	0	1	.005	2	0	10	3.412e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.023	4	-3.366e-4	1	NC	1	840.353	4
95		10	max	0	1	.004	2	0	10	3.412e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.019	4	-3.366e-4	1	NC	1	1012.066	4
97		11	max	0	1	.004	2	0	10	3.412e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.015	4	-3.366e-4	1	NC	1	1249.497	4
99		12	max	0	1	.003	2	0	10	3.412e-3	5	NC	1	NC	1





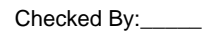
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.007	4	1.548e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-6.958e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.011	4	1.287e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-6.888e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.015	4	1.026e-5	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-6.818e-4	4	9314.337	2	NC	1
163		6	max	.001	3	.006	2	.018	4	1.849e-5	3	NC	1	NC	1
164			min	-.001	2	-.009	3	0	1	-6.748e-4	4	7465.284	2	NC	1
165		7	max	.001	3	.007	2	.022	4	3.845e-5	3	NC	3	NC	1
166			min	-.002	2	-.01	3	0	1	-6.678e-4	4	6198.456	2	NC	1
167		8	max	.002	3	.009	2	.025	4	5.841e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-6.608e-4	4	5268.111	2	NC	1
169		9	max	.002	3	.01	2	.029	4	7.837e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	1	-6.539e-4	4	4551.875	2	NC	1
171		10	max	.002	3	.012	2	.032	4	9.833e-5	3	NC	3	NC	1
172			min	-.003	2	-.014	3	0	1	-6.469e-4	4	3981.933	2	NC	1
173		11	max	.002	3	.013	2	.036	4	1.183e-4	3	NC	3	NC	1
174			min	-.003	2	-.016	3	0	1	-6.399e-4	4	3517.545	2	NC	1
175		12	max	.003	3	.015	2	.039	4	1.383e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	0	1	-6.329e-4	4	3132.604	2	NC	1
177		13	max	.003	3	.016	2	.042	4	1.582e-4	3	NC	3	NC	1
178			min	-.003	2	-.018	3	0	1	-6.259e-4	4	2809.458	2	NC	1
179		14	max	.003	3	.018	2	.045	4	1.782e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	0	1	-6.189e-4	4	2535.623	2	NC	1
181		15	max	.003	3	.02	2	.048	4	1.981e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	0	1	-6.119e-4	4	2301.935	2	NC	1
183		16	max	.003	3	.022	2	.05	4	2.181e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	0	1	-6.049e-4	4	2101.459	2	NC	1
185		17	max	.004	3	.024	2	.053	4	2.38e-4	3	NC	3	NC	1
186			min	-.005	2	-.021	3	0	1	-5.979e-4	4	1928.814	2	NC	1
187		18	max	.004	3	.026	2	.056	4	2.58e-4	3	NC	3	NC	1
188			min	-.005	2	-.022	3	-.001	1	-5.909e-4	4	1779.749	2	NC	1
189		19	max	.004	3	.028	2	.058	4	2.78e-4	3	NC	3	NC	1
190			min	-.005	2	-.023	3	-.001	1	-5.839e-4	4	1650.859	2	NC	1
191	M8	1	max	.004	1	.03	2	0	1	3.237e-3	4	NC	1	NC	1
192			min	0	3	-.023	3	-.061	4	-2.106e-4	3	NC	1	315.366	4
193		2	max	.004	1	.028	2	0	1	3.237e-3	4	NC	1	NC	1
194			min	0	3	-.022	3	-.056	4	-2.106e-4	3	NC	1	343.766	4
195		3	max	.004	1	.026	2	0	1	3.237e-3	4	NC	1	NC	1
196			min	0	3	-.021	3	-.051	4	-2.106e-4	3	NC	1	377.566	4
197		4	max	.004	1	.025	2	0	1	3.237e-3	4	NC	1	NC	1
198			min	0	3	-.019	3	-.046	4	-2.106e-4	3	NC	1	418.188	4
199		5	max	.003	1	.023	2	0	1	3.237e-3	4	NC	1	NC	1
200			min	0	3	-.018	3	-.041	4	-2.106e-4	3	NC	1	467.569	4
201		6	max	.003	1	.021	2	0	1	3.237e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.037	4	-2.106e-4	3	NC	1	528.402	4
203		7	max	.003	1	.02	2	0	1	3.237e-3	4	NC	1	NC	1
204			min	0	3	-.015	3	-.032	4	-2.106e-4	3	NC	1	604.524	4
205		8	max	.003	1	.018	2	0	1	3.237e-3	4	NC	1	NC	1
206			min	0	3	-.014	3	-.028	4	-2.106e-4	3	NC	1	701.552	4
207		9	max	.002	1	.016	2	0	1	3.237e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.023	4	-2.106e-4	3	NC	1	827.984	4
209		10	max	.002	1	.015	2	0	1	3.237e-3	4	NC	1	NC	1
210			min	0	3	-.012	3	-.019	4	-2.106e-4	3	NC	1	997.173	4
211		11	max	.002	1	.013	2	0	1	3.237e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.016	4	-2.106e-4	3	NC	1	1231.116	4
213		12	max	.002	1	.012	2	0	1	3.237e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	4.594e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.594e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.601e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-7.139e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.012	4	6.069e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-7.683e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.015	4	-4.773e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-8.227e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.018	5	-5.438e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-8.771e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.021	5	-6.103e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-9.315e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.023	5	-6.768e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-9.859e-4	4	NC	1	NC	1
285		10	max	0	3	.001	2	.026	5	-7.433e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-1.04e-3	4	NC	1	NC	1
287		11	max	0	3	.002	2	.029	5	-8.098e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-1.095e-3	4	NC	1	NC	1
289		12	max	0	3	.002	2	.032	5	-8.763e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-1.149e-3	4	NC	1	NC	1
291		13	max	0	3	.003	2	.035	5	-9.428e-6	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	1	-1.204e-3	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.038	5	-1.009e-5	10	NC	1	NC	1
294			min	-.001	2	-.007	3	-.003	1	-1.258e-3	4	NC	1	NC	1
295		15	max	.001	3	.005	2	.04	5	-1.076e-5	10	NC	1	NC	1
296			min	-.001	2	-.007	3	-.003	1	-1.312e-3	4	NC	1	NC	1
297		16	max	.001	3	.005	2	.043	5	-1.142e-5	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.004	1	-1.367e-3	4	8478.975	2	NC	1
299		17	max	.001	3	.006	2	.046	5	-1.209e-5	10	NC	1	NC	1
300			min	-.001	2	-.008	3	-.004	1	-1.421e-3	4	7239.563	2	NC	1
301		18	max	.001	3	.007	2	.048	5	-1.275e-5	10	NC	3	NC	1
302			min	-.001	2	-.008	3	-.005	1	-1.476e-3	4	6281.456	2	NC	1
303		19	max	.001	3	.008	2	.051	5	-1.342e-5	10	NC	3	NC	2
304			min	-.001	2	-.007	3	-.005	1	-1.53e-3	4	5532.924	2	9306.688	1
305	M12	1	max	.002	1	.009	2	.004	1	4.059e-3	4	NC	1	NC	2
306			min	0	3	-.007	3	-.056	5	1.406e-5	10	NC	1	343.313	5
307		2	max	.001	1	.008	2	.004	1	4.059e-3	4	NC	1	NC	2
308			min	0	3	-.007	3	-.052	5	1.406e-5	10	NC	1	374.222	5
309		3	max	.001	1	.008	2	.003	1	4.059e-3	4	NC	1	NC	2
310			min	0	3	-.007	3	-.047	5	1.406e-5	10	NC	1	411.006	5
311		4	max	.001	1	.007	2	.003	1	4.059e-3	4	NC	1	NC	2
312			min	0	3	-.006	3	-.042	5	1.406e-5	10	NC	1	455.214	5
313		5	max	.001	1	.007	2	.003	1	4.059e-3	4	NC	1	NC	2
314			min	0	3	-.006	3	-.038	5	1.406e-5	10	NC	1	508.952	5
315		6	max	.001	1	.006	2	.002	1	4.059e-3	4	NC	1	NC	2
316			min	0	3	-.005	3	-.034	5	1.406e-5	10	NC	1	575.152	5
317		7	max	.001	1	.006	2	.002	1	4.059e-3	4	NC	1	NC	2
318			min	0	3	-.005	3	-.029	5	1.406e-5	10	NC	1	657.988	5
319		8	max	0	1	.005	2	.002	1	4.059e-3	4	NC	1	NC	1
320			min	0	3	-.005	3	-.025	5	1.406e-5	10	NC	1	763.572	5
321		9	max	0	1	.005	2	.002	1	4.059e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.021	5	1.406e-5	10	NC	1	901.149	5
323		10	max	0	1	.004	2	.001	1	4.059e-3	4	NC	1	NC	1
324			min	0	3	-.004	3	-.018	5	1.406e-5	10	NC	1	1085.25	5
325		11	max	0	1	.004	2	.001	1	4.059e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.014	5	1.406e-5	10	NC	1	1339.806	5
327		12	max	0	1	.003	2	0	1	4.059e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.011	5	1.406e-5	10	NC	1	1706.559	5
329		max	0	1	.003	2	0	1	4.059e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.009	5	1.406e-5	10	NC	1	2263.802	5
331		max	0	1	.002	2	0	1	4.059e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.006	5	1.406e-5	10	NC	1	3173.021	5
333		max	0	1	.002	2	0	1	4.059e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.004	5	1.406e-5	10	NC	1	4813.588	5
335		max	0	1	.001	2	0	1	4.059e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.002	5	1.406e-5	10	NC	1	8263.917	5
337		max	0	1	0	2	0	1	4.059e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.001	5	1.406e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	4.059e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	1.406e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	4.059e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.406e-5	10	NC	1	NC	1
343	M1	max	.007	3	.023	3	.006	5	8.129e-3	1	NC	1	NC	1
344		min	-.007	2	-.02	2	-.002	1	-1.106e-2	3	NC	1	NC	1
345		max	.007	3	.013	3	.008	5	3.912e-3	2	NC	4	NC	1
346		min	-.007	2	-.011	2	-.004	1	-5.456e-3	3	5020.156	3	NC	1
347		max	.007	3	.004	3	.011	5	3.136e-4	5	NC	4	NC	1
348		min	-.007	2	-.003	2	-.005	1	-2.371e-4	1	2603.984	3	9558.06	5
349		max	.007	3	.004	2	.014	5	3.14e-4	5	NC	4	NC	1
350		min	-.007	2	-.003	3	-.006	1	-1.998e-4	1	1860.605	3	6008.356	5
351		max	.007	3	.01	2	.017	5	3.144e-4	5	NC	4	NC	1
352		min	-.007	2	-.01	3	-.006	1	-1.624e-4	1	1494.122	2	4285.781	5
353		max	.007	3	.016	2	.02	5	3.149e-4	5	NC	4	NC	1
354		min	-.007	2	-.015	3	-.005	1	-1.251e-4	1	1271.411	2	3284.416	5
355		max	.007	3	.02	2	.024	5	3.153e-4	5	NC	4	NC	1
356		min	-.007	2	-.018	3	-.005	1	-8.775e-5	1	1134.285	2	2638.405	5
357		max	.007	3	.023	2	.027	5	3.158e-4	5	NC	4	NC	1
358		min	-.007	2	-.021	3	-.004	1	-5.042e-5	1	1048.538	2	2192.182	5
359		max	.007	3	.025	2	.031	5	3.162e-4	5	NC	5	NC	1
360		min	-.007	2	-.022	3	-.003	1	-1.791e-5	9	998.036	2	1867.817	4
361		max	.007	3	.026	2	.035	5	3.222e-4	4	NC	5	NC	1
362		min	-.007	2	-.023	3	-.001	1	3.825e-6	10	975.103	2	1608.092	4
363		max	.007	3	.025	2	.039	4	3.312e-4	4	NC	4	NC	1
364		min	-.007	2	-.022	3	0	9	5.321e-6	10	977.007	2	1411.217	4
365		max	.007	3	.024	2	.043	4	3.401e-4	4	NC	4	NC	1
366		min	-.007	2	-.02	3	0	10	6.817e-6	10	1004.93	2	1258.688	4
367		max	.007	3	.021	2	.047	4	3.491e-4	4	NC	4	NC	1
368		min	-.007	2	-.017	3	0	10	8.313e-6	10	1064.598	2	1138.547	4
369		max	.007	3	.016	2	.05	4	3.581e-4	4	NC	4	NC	1
370		min	-.007	2	-.013	3	0	10	9.809e-6	10	1169.157	2	1042.775	4
371		max	.007	3	.011	2	.054	4	3.67e-4	4	NC	4	NC	1
372		min	-.007	2	-.009	3	0	10	1.131e-5	10	1347.404	2	965.844	4
373		max	.007	3	.004	2	.057	4	5.753e-4	4	NC	4	NC	1
374		min	-.007	2	-.003	3	0	10	1.243e-5	10	1669.581	2	903.85	4
375		max	.007	3	.003	3	.06	4	5.526e-3	4	NC	4	NC	1
376		min	-.007	2	-.005	2	0	10	-7.413e-7	9	2358.898	2	854.044	4
377		max	.007	3	.01	3	.063	4	5.473e-3	2	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-2.601e-3	3	4567.043	2	814.28	4
379		max	.007	3	.018	3	.065	4	1.102e-2	2	NC	1	NC	1
380		min	-.007	2	-.025	2	-.001	1	-5.297e-3	3	NC	1	783.962	4
381	M5	max	.021	3	.072	3	.006	5	1.163e-5	4	NC	1	NC	1
382		min	-.025	2	-.064	2	-.002	1	5.238e-8	11	NC	1	NC	1
383		max	.021	3	.041	3	.008	5	1.555e-4	5	NC	4	NC	1
384		min	-.025	2	-.036	2	-.002	1	-3.355e-5	1	1568.618	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.021	3	.013	3	.011	5	2.971e-4	5	NC	5	NC	1
386		min	-.025	2	-.009	2	-.002	1	-6.651e-5	1	813.969	3	NC	1
387	4	max	.021	3	.014	2	.014	5	3.088e-4	5	NC	5	NC	1
388		min	-.025	2	-.011	3	-.002	1	-6.299e-5	1	577.603	2	NC	1
389	5	max	.021	3	.034	2	.017	5	3.204e-4	5	NC	5	NC	1
390		min	-.025	2	-.031	3	-.002	1	-5.947e-5	1	457.694	2	NC	1
391	6	max	.021	3	.051	2	.021	5	3.321e-4	5	NC	5	NC	1
392		min	-.025	2	-.047	3	-.002	1	-5.595e-5	1	389.234	2	NC	1
393	7	max	.021	3	.065	2	.025	5	3.437e-4	5	NC	5	NC	1
394		min	-.025	2	-.058	3	-.002	1	-5.243e-5	1	347.063	2	NC	1
395	8	max	.021	3	.075	2	.029	5	3.554e-4	5	NC	5	NC	1
396		min	-.025	2	-.066	3	-.002	1	-4.891e-5	1	320.672	2	NC	1
397	9	max	.021	3	.081	2	.033	5	3.67e-4	5	NC	5	NC	1
398		min	-.025	2	-.071	3	-.001	1	-4.538e-5	1	305.099	2	NC	1
399	10	max	.021	3	.084	2	.037	5	3.787e-4	5	NC	5	NC	1
400		min	-.025	2	-.072	3	-.001	1	-4.186e-5	1	297.983	2	NC	1
401	11	max	.021	3	.083	2	.041	5	3.903e-4	5	NC	5	NC	1
402		min	-.025	2	-.069	3	-.001	1	-3.834e-5	1	298.48	2	NC	1
403	12	max	.021	3	.077	2	.045	5	4.02e-4	5	NC	5	NC	1
404		min	-.025	2	-.063	3	-.001	1	-3.482e-5	1	306.947	2	NC	1
405	13	max	.021	3	.068	2	.049	4	4.136e-4	5	NC	5	NC	1
406		min	-.025	2	-.055	3	-.001	1	-3.13e-5	1	325.13	2	NC	1
407	14	max	.021	3	.054	2	.052	4	4.252e-4	5	NC	5	NC	1
408		min	-.025	2	-.043	3	-.001	1	-2.778e-5	1	357.053	2	NC	1
409	15	max	.021	3	.035	2	.056	4	4.369e-4	5	NC	5	NC	1
410		min	-.025	2	-.028	3	-.001	1	-2.492e-5	9	411.533	2	NC	1
411	16	max	.021	3	.012	2	.059	4	6.446e-4	5	NC	5	NC	1
412		min	-.025	2	-.01	3	-.001	1	-2.381e-5	9	510.107	2	NC	1
413	17	max	.021	3	.01	3	.061	4	5.537e-3	4	NC	5	NC	1
414		min	-.025	2	-.016	2	0	1	-7.524e-5	1	721.502	2	NC	1
415	18	max	.021	3	.032	3	.064	4	2.842e-3	4	NC	4	NC	1
416		min	-.025	2	-.048	2	0	1	-3.849e-5	1	1397.687	2	NC	1
417	19	max	.021	3	.055	3	.065	4	4.058e-6	5	NC	1	NC	1
418		min	-.025	2	-.083	2	0	1	-6.179e-7	3	NC	1	NC	1
419	M9	1	max	.007	.022	.005	.005	5	1.107e-2	3	NC	1	NC	1
420		min	-.007	2	-.02	2	-.002	1	-8.129e-3	1	NC	1	NC	1
421	2	max	.007	3	.013	.005	.005	5	5.471e-3	3	NC	4	NC	1
422		min	-.007	2	-.011	2	0	9	-3.98e-3	1	5021.918	3	NC	1
423	3	max	.007	3	.004	.005	.005	4	9.139e-5	1	NC	4	NC	1
424		min	-.007	2	-.003	2	0	3	-3.389e-5	5	2604.917	3	NC	1
425	4	max	.007	3	.004	.006	.006	4	6.014e-5	1	NC	4	NC	1
426		min	-.007	2	-.004	3	-.001	3	-3.365e-5	5	1861.255	3	NC	1
427	5	max	.007	3	.01	.007	.007	4	3.045e-5	2	NC	4	NC	1
428		min	-.007	2	-.01	3	-.002	3	-3.836e-5	3	1494.488	2	NC	1
429	6	max	.007	3	.016	.01	.01	4	1.965e-5	2	NC	4	NC	1
430		min	-.007	2	-.015	3	-.003	3	-4.547e-5	3	1271.737	2	9336.501	4
431	7	max	.007	3	.02	.012	.012	4	8.855e-6	2	NC	4	NC	1
432		min	-.007	2	-.019	3	-.003	3	-5.257e-5	3	1134.588	2	6063.991	4
433	8	max	.007	3	.023	.015	.015	4	-7.025e-7	10	NC	5	NC	1
434		min	-.007	2	-.021	3	-.004	3	-6.487e-5	1	1048.831	2	4300.442	4
435	9	max	.007	3	.025	.019	.019	4	-2.206e-6	10	NC	5	NC	1
436		min	-.007	2	-.022	3	-.004	3	-9.612e-5	1	998.325	2	3238.444	4
437	10	max	.007	3	.026	.023	.023	5	-3.71e-6	10	NC	5	NC	1
438		min	-.007	2	-.023	3	-.004	3	-1.274e-4	1	975.395	2	2547.785	4
439	11	max	.007	3	.025	.027	.027	5	-5.214e-6	10	NC	5	NC	1
440		min	-.007	2	-.022	3	-.004	3	-1.586e-4	1	977.308	2	2072.555	4
441	12	max	.007	3	.024	.032	.032	5	-6.717e-6	10	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.007	2	-.02	3	-.004	3	-1.899e-4	1	1005.248	2	1730.485	5
443		13	max	.007	3	.021	2	.037	5	-8.221e-6	10	NC	4	NC	1
444			min	-.007	2	-.017	3	-.004	1	-2.211e-4	1	1064.942	2	1470.124	5
445		14	max	.007	3	.016	2	.042	5	-9.725e-6	10	NC	4	NC	1
446			min	-.007	2	-.014	3	-.005	1	-2.524e-4	1	1169.542	2	1275.1	5
447		15	max	.007	3	.011	2	.047	5	-1.123e-5	10	NC	4	NC	1
448			min	-.007	2	-.009	3	-.005	1	-2.836e-4	1	1347.85	2	1125.503	5
449		16	max	.007	3	.004	2	.052	5	1.832e-4	5	NC	4	NC	1
450			min	-.007	2	-.003	3	-.005	1	-3.079e-4	1	1670.128	2	1008.612	5
451		17	max	.007	3	.003	3	.056	5	5.49e-3	5	NC	4	NC	1
452			min	-.007	2	-.005	2	-.004	1	-1.659e-4	1	2359.615	2	915.791	4
453		18	max	.007	3	.01	3	.061	5	2.675e-3	5	NC	4	NC	1
454			min	-.007	2	-.015	2	-.003	1	-5.474e-3	2	4568.387	2	836.333	4
455		19	max	.007	3	.018	3	.065	4	5.296e-3	3	NC	1	NC	1
456			min	-.007	2	-.025	2	0	1	-1.102e-2	2	NC	1	771.333	4
457	M13	1	max	.002	1	.022	3	.007	3	3.626e-3	3	NC	1	NC	1
458			min	-.005	5	-.02	2	-.007	2	-3.277e-3	2	NC	1	NC	1
459		2	max	.002	1	.09	3	.006	9	4.518e-3	3	NC	4	NC	1
460			min	-.005	5	-.068	2	-.005	2	-4.098e-3	2	1682.812	3	NC	1
461		3	max	.002	1	.146	3	.017	1	5.409e-3	3	NC	5	NC	2
462			min	-.005	5	-.109	1	-.004	10	-4.919e-3	2	919.581	3	5530.929	1
463		4	max	.002	1	.183	3	.025	1	6.301e-3	3	NC	5	NC	2
464			min	-.005	5	-.137	1	-.004	10	-5.741e-3	2	708.642	3	3883.001	1
465		5	max	.002	1	.197	3	.028	1	7.192e-3	3	NC	5	NC	2
466			min	-.005	5	-.148	1	-.005	10	-6.562e-3	2	652.577	3	3510.267	1
467		6	max	.002	1	.188	3	.025	1	8.084e-3	3	NC	5	NC	2
468			min	-.005	5	-.142	2	-.007	10	-7.383e-3	2	687.876	3	3957.726	1
469		7	max	.002	1	.161	3	.016	9	8.975e-3	3	NC	5	NC	2
470			min	-.005	5	-.124	2	-.009	2	-8.204e-3	2	824.164	3	5996.356	1
471		8	max	.002	1	.123	3	.016	3	9.867e-3	3	NC	5	NC	1
472			min	-.006	5	-.099	2	-.016	2	-9.026e-3	2	1130.156	3	NC	1
473		9	max	.002	1	.088	3	.019	3	1.076e-2	3	NC	4	NC	1
474			min	-.006	5	-.075	2	-.022	2	-9.847e-3	2	1731.986	3	7634.805	2
475		10	max	.002	1	.072	3	.021	3	1.165e-2	3	NC	4	NC	4
476			min	-.006	5	-.064	2	-.025	2	-1.067e-2	2	2295.319	3	6456.484	2
477		11	max	.002	1	.088	3	.023	3	1.076e-2	3	NC	4	NC	1
478			min	-.006	5	-.075	2	-.022	2	-9.847e-3	2	1731.985	3	7031.349	3
479		12	max	.002	1	.123	3	.024	3	9.87e-3	3	NC	5	NC	1
480			min	-.006	5	-.099	2	-.016	2	-9.026e-3	2	1130.155	3	6748.83	3
481		13	max	.002	1	.161	3	.023	3	8.979e-3	3	NC	5	NC	2
482			min	-.006	5	-.124	2	-.009	2	-8.204e-3	2	824.163	3	5968.689	1
483		14	max	.002	1	.188	3	.025	1	8.089e-3	3	NC	5	NC	2
484			min	-.006	5	-.142	2	-.007	10	-7.383e-3	2	687.876	3	3953.785	1
485		15	max	.002	1	.197	3	.028	1	7.199e-3	3	NC	5	NC	2
486			min	-.006	5	-.148	1	-.005	10	-6.562e-3	2	652.576	3	3514.649	1
487		16	max	.002	1	.183	3	.025	1	6.309e-3	3	NC	5	NC	2
488			min	-.006	5	-.137	1	-.004	10	-5.741e-3	2	708.642	3	3896.103	1
489		17	max	.002	1	.146	3	.016	1	5.419e-3	3	NC	5	NC	2
490			min	-.006	5	-.109	1	-.004	10	-4.919e-3	2	919.58	3	5564.74	1
491		18	max	.002	1	.09	3	.009	3	4.528e-3	3	NC	4	NC	1
492			min	-.006	5	-.068	2	-.005	2	-4.098e-3	2	1682.811	3	NC	1
493		19	max	.002	1	.023	3	.007	3	3.638e-3	3	NC	1	NC	1
494			min	-.006	5	-.02	2	-.007	2	-3.277e-3	2	NC	1	NC	1
495	M16	1	max	0	1	.018	3	.007	3	4.004e-3	2	NC	1	NC	1
496			min	-.065	4	-.025	2	-.007	2	-2.8e-3	3	NC	1	NC	1
497		2	max	0	1	.05	3	.009	3	5.015e-3	2	NC	4	NC	1
498			min	-.065	4	-.093	2	-.005	2	-3.467e-3	3	1688.491	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	0	1	.077	3	.016	14	6.025e-3	2	NC	5	NC	2
500		min	-.065	4	-.149	2	-.004	10	-4.135e-3	3	921.068	2	5544.866	1
501	4	max	0	1	.096	3	.025	1	7.035e-3	2	NC	5	NC	2
502		min	-.065	4	-.186	2	-.004	10	-4.802e-3	3	707.567	2	3893.3	1
503	5	max	0	1	.104	3	.028	1	8.046e-3	2	NC	5	NC	2
504		min	-.065	4	-.201	2	-.005	10	-5.469e-3	3	648.256	2	3521.438	1
505	6	max	0	1	.102	3	.024	1	9.056e-3	2	NC	5	NC	2
506		min	-.065	4	-.193	2	-.007	10	-6.137e-3	3	677.593	2	3975.326	1
507	7	max	0	1	.092	3	.022	3	1.007e-2	2	NC	5	NC	2
508		min	-.065	4	-.168	2	-.009	2	-6.804e-3	3	800.16	2	6044.306	1
509	8	max	0	1	.077	3	.022	3	1.108e-2	2	NC	5	NC	1
510		min	-.065	4	-.132	2	-.016	2	-7.472e-3	3	1068.535	2	7449.024	3
511	9	max	0	1	.062	3	.022	3	1.209e-2	2	NC	4	NC	1
512		min	-.065	4	-.098	2	-.022	2	-8.139e-3	3	1561.959	2	7630.698	3
513	10	max	0	1	.055	3	.021	3	1.31e-2	2	NC	4	NC	4
514		min	-.065	4	-.083	2	-.025	2	-8.806e-3	3	1985.948	2	6454.384	2
515	11	max	0	1	.062	3	.019	3	1.209e-2	2	NC	4	NC	1
516		min	-.065	4	-.098	2	-.022	2	-8.138e-3	3	1561.959	2	7631.062	2
517	12	max	0	1	.077	3	.018	3	1.108e-2	2	NC	5	NC	1
518		min	-.065	4	-.132	2	-.016	2	-7.469e-3	3	1068.535	2	9786.382	3
519	13	max	0	1	.092	3	.017	3	1.007e-2	2	NC	5	NC	2
520		min	-.065	4	-.168	2	-.009	2	-6.801e-3	3	800.16	2	6036.831	1
521	14	max	0	1	.102	3	.024	1	9.057e-3	2	NC	5	NC	2
522		min	-.065	4	-.193	2	-.007	10	-6.132e-3	3	677.593	2	3981.322	1
523	15	max	.001	1	.104	3	.028	1	8.047e-3	2	NC	5	NC	2
524		min	-.065	4	-.201	2	-.005	10	-5.463e-3	3	648.256	2	3533.699	1
525	16	max	.001	1	.096	3	.025	1	7.036e-3	2	NC	5	NC	2
526		min	-.065	4	-.186	2	-.004	10	-4.795e-3	3	707.567	2	3915.137	1
527	17	max	.001	1	.077	3	.016	1	6.026e-3	2	NC	5	NC	2
528		min	-.065	4	-.149	2	-.005	5	-4.126e-3	3	921.068	2	5592.584	1
529	18	max	.001	1	.05	3	.008	3	5.016e-3	2	NC	4	NC	1
530		min	-.065	4	-.093	2	-.005	2	-3.457e-3	3	1688.491	2	NC	1
531	19	max	.001	1	.018	3	.007	3	4.006e-3	2	NC	1	NC	1
532		min	-.065	4	-.025	2	-.007	2	-2.789e-3	3	NC	1	NC	1
533	M15	max	0	1	0	1	0	1	3.625e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.65e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.005	4	8.01e-4	3	NC	1	NC	1
536		min	0	4	-.003	1	0	3	-5.816e-4	5	NC	1	NC	1
537	3	max	0	3	0	5	.011	4	1.24e-3	3	NC	1	NC	1
538		min	-.001	4	-.006	1	-.003	3	-9.397e-4	2	NC	1	6431.509	4
539	4	max	0	3	.001	5	.016	4	1.678e-3	3	NC	5	NC	9
540		min	-.002	4	-.009	1	-.007	3	-1.384e-3	2	7227.969	2	4166.985	4
541	5	max	0	3	.002	5	.022	4	2.117e-3	3	NC	5	NC	9
542		min	-.002	4	-.012	1	-.011	3	-1.828e-3	2	5640.057	2	3135.122	4
543	6	max	0	3	.002	5	.026	4	2.555e-3	3	NC	5	8906.275	9
544		min	-.003	4	-.014	1	-.016	3	-2.272e-3	2	4746.704	2	2586.682	4
545	7	max	0	3	.002	5	.03	4	2.994e-3	3	NC	5	7012.52	9
546		min	-.003	4	-.016	1	-.021	3	-2.716e-3	2	4209.47	2	2281.246	4
547	8	max	0	3	.003	5	.032	4	3.432e-3	3	NC	5	5813.921	9
548		min	-.004	4	-.017	1	-.025	3	-3.16e-3	2	3887.049	2	1921.997	3
549	9	max	0	3	.003	5	.033	4	3.871e-3	3	NC	5	5026.191	9
550		min	-.004	4	-.018	1	-.03	3	-3.604e-3	2	3713.503	2	1654.611	3
551	10	max	0	3	.003	5	.032	4	4.309e-3	3	NC	5	4505.448	9
552		min	-.005	4	-.019	1	-.033	3	-4.048e-3	2	3658.601	2	1478.127	3
553	11	max	0	3	.004	5	.031	1	4.748e-3	3	NC	5	4175.909	9
554		min	-.005	4	-.018	1	-.035	3	-4.491e-3	2	3713.503	2	1366.143	3
555	12	max	0	3	.004	5	.032	1	5.186e-3	3	NC	5	4777.038	15



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

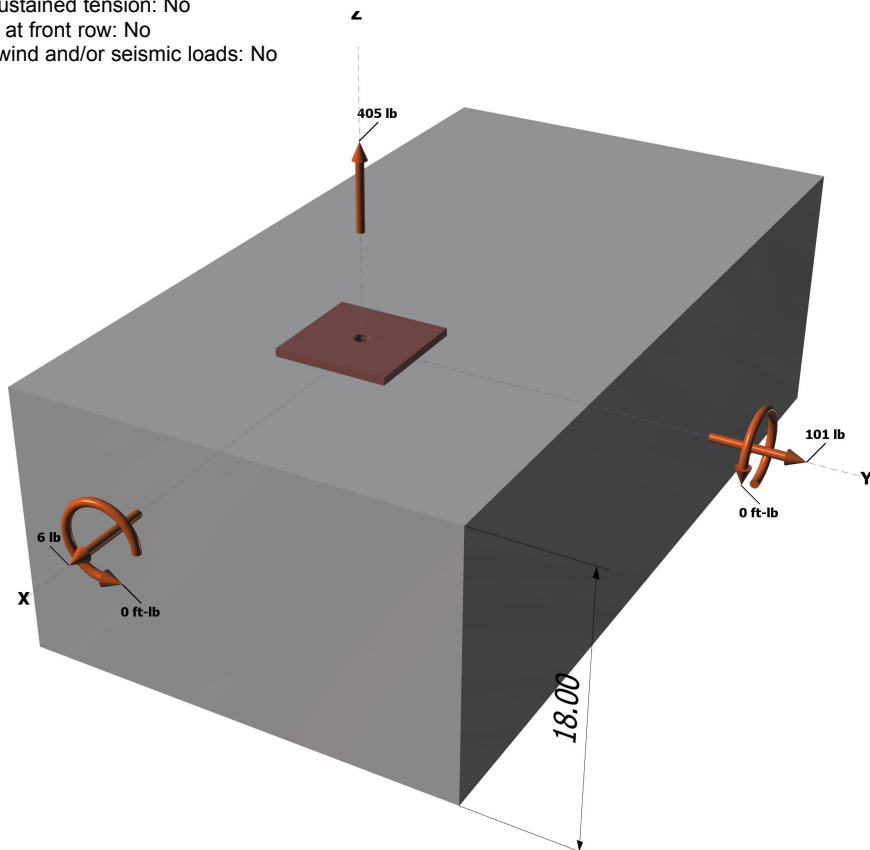
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

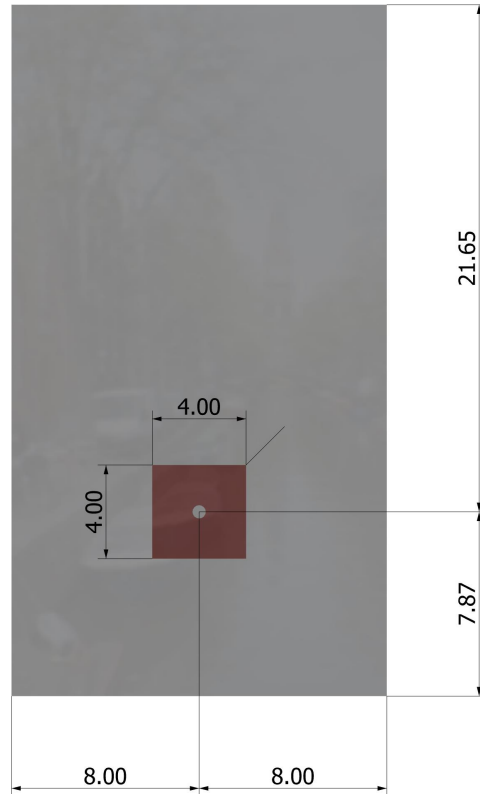
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

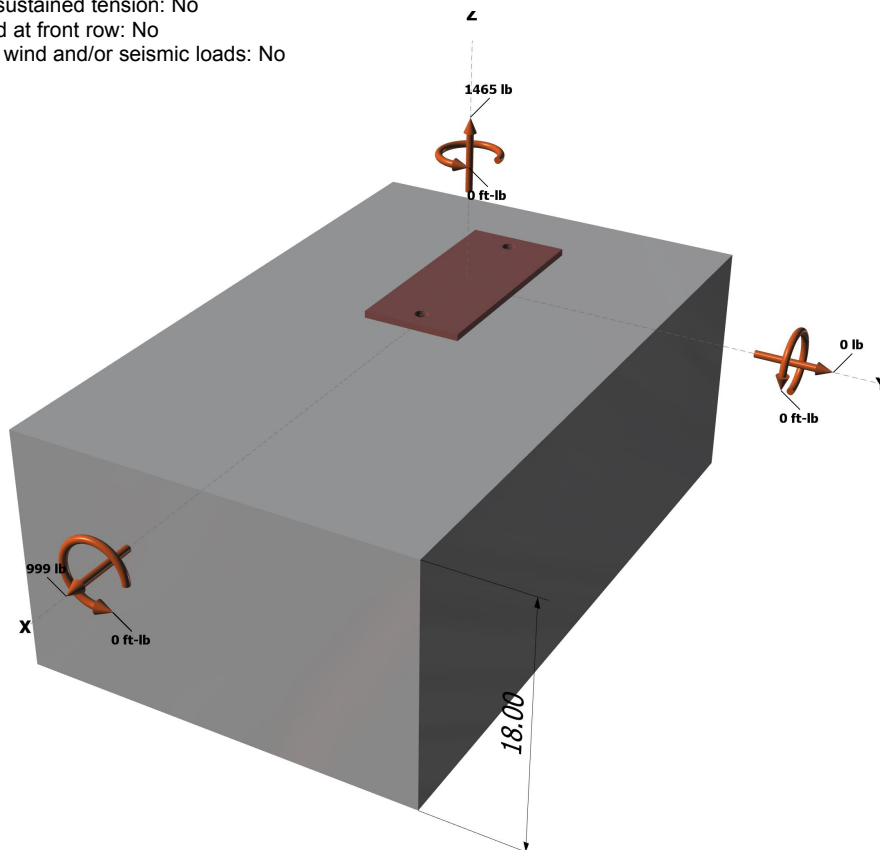
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

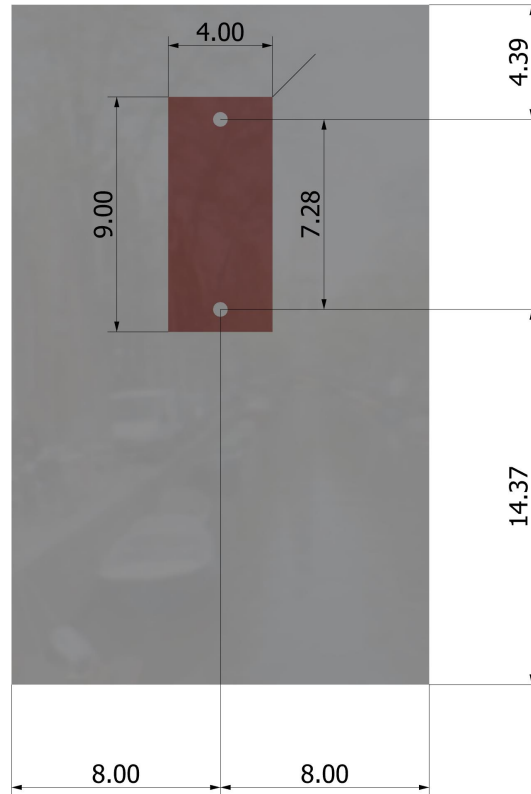
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.