

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	160 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 40.19 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.914 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	7%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.319 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.697 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	10%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.158 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

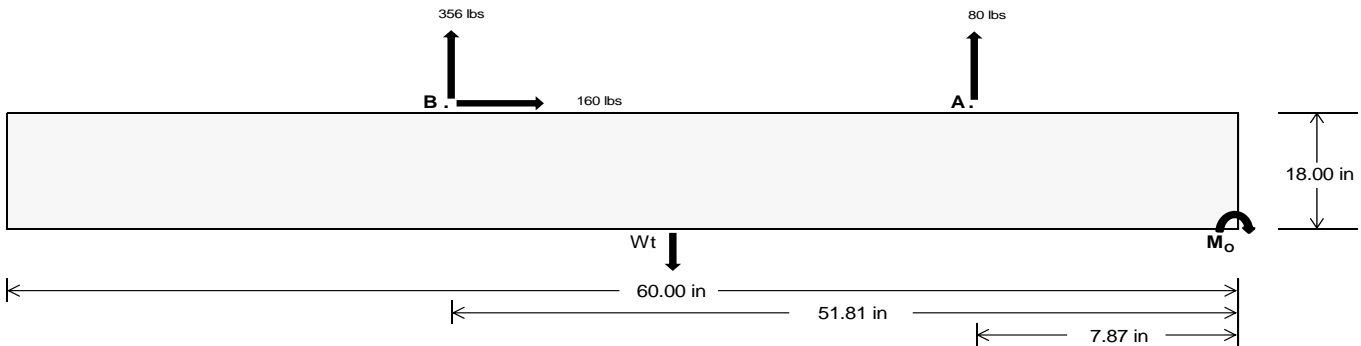
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	351.44	1544.28	k
Compressive Load =	1188.80	1004.52	k
Lateral Load =	20.08	691.55	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 21928.5$ in-lbs
Resisting Force Required = 730.95 lbs
S.F. = 1.67
Weight Required = 1218.25 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 159.57 lbs
Friction = 0.4
Weight Required = 398.92 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 159.57 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$

Ballast Width			
20 in	21 in	22 in	23 in
1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	362 lbs	362 lbs	362 lbs	362 lbs	473 lbs	473 lbs	473 lbs	473 lbs	598 lbs	598 lbs	598 lbs	598 lbs	-160 lbs	-160 lbs	-160 lbs	-160 lbs
F_B	256 lbs	256 lbs	256 lbs	256 lbs	410 lbs	410 lbs	410 lbs	410 lbs	480 lbs	480 lbs	480 lbs	480 lbs	-711 lbs	-711 lbs	-711 lbs	-711 lbs
F_V	22 lbs	22 lbs	22 lbs	22 lbs	280 lbs	280 lbs	280 lbs	280 lbs	225 lbs	225 lbs	225 lbs	225 lbs	-319 lbs	-319 lbs	-319 lbs	-319 lbs
P_{total}	2431 lbs	2521 lbs	2612 lbs	2703 lbs	2695 lbs	2786 lbs	2877 lbs	2967 lbs	2890 lbs	2981 lbs	3071 lbs	3162 lbs	216 lbs	271 lbs	325 lbs	379 lbs
M	235 lbs-ft	235 lbs-ft	235 lbs-ft	235 lbs-ft	545 lbs-ft	545 lbs-ft	545 lbs-ft	545 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	568 lbs-ft	519 lbs-ft	519 lbs-ft	519 lbs-ft	519 lbs-ft
e	0.10 ft	0.09 ft	0.09 ft	0.09 ft	0.20 ft	0.20 ft	0.19 ft	0.18 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	2.40 ft	1.92 ft	1.60 ft	1.37 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	257.9 psf	256.0 psf	254.2 psf	252.6 psf	243.7 psf	242.5 psf	241.4 psf	240.0 psf	262.7 psf	260.6 psf	258.8 psf	258.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	325.5 psf	320.3 psf	315.7 psf	311.4 psf	401.9 psf	393.1 psf	385.2 psf	377.9 psf	428.7 psf	418.6 psf	409.5 psf	401.1 psf	874.2 psf	177.3 psf	130.9 psf	116.6 psf

Maximum Bearing Pressure = 874 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

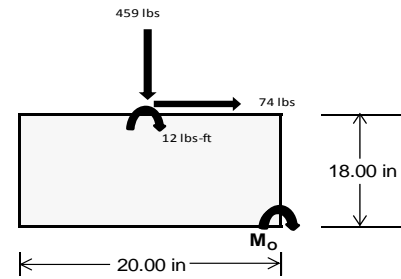
Overturning Check

$M_o = 260.3 \text{ ft-lbs}$
 Resisting Force Required = 312.33 lbs
 S.F. = 1.67
 Weight Required = 520.56 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	105 lbs	59 lbs	50 lbs	217 lbs	459 lbs	175 lbs	71 lbs	-26 lbs	18 lbs
F_v	12 lbs	98 lbs	12 lbs	8 lbs	74 lbs	9 lbs	12 lbs	98 lbs	12 lbs
P_{total}	2349 lbs	2303 lbs	2294 lbs	2353 lbs	2595 lbs	2311 lbs	727 lbs	630 lbs	674 lbs
M	33 lbs-ft	163 lbs-ft	33 lbs-ft	24 lbs-ft	122 lbs-ft	25 lbs-ft	33 lbs-ft	163 lbs-ft	33 lbs-ft
e	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.26 ft	0.05 ft
$L/6$	0.28 ft	1.53 ft	1.64 ft	1.65 ft	1.57 ft	1.64 ft	1.58 ft	1.15 ft	1.57 ft
f_{min}	267.9 sqft	205.9 sqft	260.9 sqft	272.1 sqft	258.5 sqft	266.4 sqft	73.2 sqft	5.2 sqft	66.4 sqft
f_{max}	296.0 psf	346.7 psf	289.7 psf	292.6 psf	364.3 psf	288.2 psf	101.4 psf	146.0 psf	95.3 psf



Maximum Bearing Pressure = 364 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

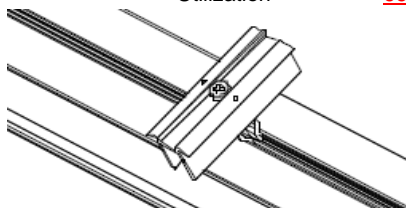
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

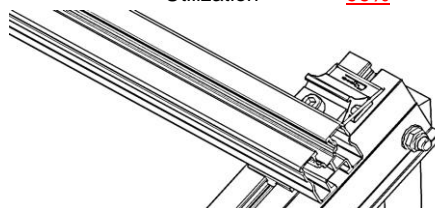
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.803 k
Allowable Uplift =	1.214 k
Utilization =	<u>66%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.075 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

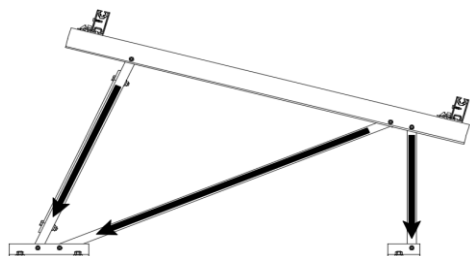
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.914 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

Diagonal Strut

Maximum Axial Load =	0.319 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.089 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Bracing

Maximum Axial Load =	0.158 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.05 in
	<u>0.05 ≤ 0.591, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$117.177$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$121.682$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.271 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.43 \\
 &20.5689 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.43 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.9 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.468 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.41804$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77853$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 13.5508 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 13.55 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

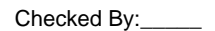
$$0.50 \text{ in}^2$$

$$P_{\max} = 6.80 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 20° 160mph 30psf 3.75ft 7-10Pa Page 20



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	224.444	1	.082	2	.093	1	0	10	0	4	0	15
30		min	-355.214	3	.014	15	-.473	5	0	4	0	3	0	6
31	16	max	224.551	1	.05	2	.093	1	0	10	0	4	0	15
32		min	-355.134	3	-.005	3	-.57	5	0	4	0	3	0	6
33	17	max	224.657	1	.018	2	.093	1	0	10	0	4	0	15
34		min	-355.054	3	-.029	3	-.666	5	0	4	0	3	0	6
35	18	max	224.764	1	-.015	2	.093	1	0	10	0	14	0	15
36		min	-354.974	3	-.058	4	-.762	5	0	4	0	3	0	6
37	19	max	224.87	1	-.025	15	.093	1	0	10	0	1	0	15
38		min	-354.894	3	-.099	4	-.859	5	0	4	0	3	0	6
39	M3	1	max	101.163	2	1.796	.007	10	0	5	0	4	0	6
40		min	-88.038	3	.421	15	-1.338	4	0	1	0	10	0	15
41	2	max	101.095	2	1.618	6	.007	10	0	5	0	1	0	6
42		min	-88.089	3	.379	15	-1.204	4	0	1	0	10	0	15
43	3	max	101.027	2	1.44	6	.007	10	0	5	0	1	0	2
44		min	-88.14	3	.337	15	-1.071	4	0	1	0	5	0	3
45	4	max	100.96	2	1.263	6	.007	10	0	5	0	1	0	15
46		min	-88.191	3	.296	15	-.937	4	0	1	0	5	0	4
47	5	max	100.892	2	1.085	6	.007	10	0	5	0	1	0	15
48		min	-88.242	3	.254	15	-.804	4	0	1	0	5	0	4
49	6	max	100.824	2	.908	6	.007	10	0	5	0	1	0	15
50		min	-88.293	3	.212	15	-.67	4	0	1	0	5	0	4
51	7	max	100.756	2	.73	6	.007	10	0	5	0	1	0	15
52		min	-88.344	3	.17	15	-.536	4	0	1	0	5	0	4
53	8	max	100.688	2	.552	6	.007	10	0	5	0	1	0	15
54		min	-88.395	3	.128	15	-.403	4	0	1	0	5	0	4
55	9	max	100.62	2	.375	6	.007	10	0	5	0	1	0	15
56		min	-88.445	3	.087	15	-.269	4	0	1	0	5	-.001	4
57	10	max	100.552	2	.197	6	.007	10	0	5	0	1	0	15
58		min	-88.496	3	.045	15	-.136	4	0	1	0	5	-.001	4
59	11	max	100.485	2	.037	2	.026	5	0	5	0	1	0	15
60		min	-88.547	3	-.003	3	-.125	1	0	1	0	5	-.001	4
61	12	max	100.417	2	-.039	15	.16	5	0	5	0	1	0	15
62		min	-88.598	3	-.159	4	-.125	1	0	1	0	5	-.001	4
63	13	max	100.349	2	-.08	15	.293	5	0	5	0	1	0	15
64		min	-88.649	3	-.336	4	-.125	1	0	1	0	5	-.001	4
65	14	max	100.281	2	-.122	15	.427	5	0	5	0	9	0	15
66		min	-88.7	3	-.514	4	-.125	1	0	1	0	5	-.001	4
67	15	max	100.213	2	-.164	15	.56	5	0	5	0	10	0	15
68		min	-88.751	3	-.692	4	-.125	1	0	1	0	4	0	4
69	16	max	100.145	2	-.206	15	.694	5	0	5	0	10	0	15
70		min	-88.802	3	-.869	4	-.125	1	0	1	0	4	0	4
71	17	max	100.077	2	-.247	15	.828	5	0	5	0	10	0	15
72		min	-88.853	3	-1.047	4	-.125	1	0	1	0	4	0	4
73	18	max	100.009	2	-.289	15	.961	5	0	5	0	10	0	15
74		min	-88.903	3	-1.224	4	-.125	1	0	1	0	4	0	4
75	19	max	99.942	2	-.331	15	1.095	5	0	5	0	5	0	1
76		min	-88.954	3	-1.402	4	-.125	1	0	1	0	1	0	1
77	M4	1	max	306.402	1	0	.033	10	0	1	0	5	0	1
78		min	-75.687	3	0	1	-14.246	4	0	1	0	2	0	1
79	2	max	306.466	1	0	1	.033	10	0	1	0	10	0	1
80		min	-75.639	3	0	1	-14.302	4	0	1	-.001	4	0	1
81	3	max	306.531	1	0	1	.033	10	0	1	0	10	0	1
82		min	-75.59	3	0	1	-14.358	4	0	1	-.003	4	0	1
83	4	max	306.596	1	0	1	.033	10	0	1	0	10	0	1
84		min	-75.542	3	0	1	-14.414	4	0	1	-.004	4	0	1
85	5	max	306.661	1	0	1	.033	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-75.493	3	0	1	-14.47	4	0	1	-.005	4	0	1
87		6	max	306.725	1	0	1	.033	10	0	1	0	10	0	1
88			min	-75.445	3	0	1	-14.527	4	0	1	-.006	4	0	1
89		7	max	306.79	1	0	1	.033	10	0	1	0	10	0	1
90			min	-75.396	3	0	1	-14.583	4	0	1	-.008	4	0	1
91		8	max	306.855	1	0	1	.033	10	0	1	0	10	0	1
92			min	-75.348	3	0	1	-14.639	4	0	1	-.009	4	0	1
93		9	max	306.919	1	0	1	.033	10	0	1	0	10	0	1
94			min	-75.299	3	0	1	-14.695	4	0	1	-.01	4	0	1
95		10	max	306.984	1	0	1	.033	10	0	1	0	10	0	1
96			min	-75.25	3	0	1	-14.751	4	0	1	-.012	4	0	1
97		11	max	307.049	1	0	1	.033	10	0	1	0	10	0	1
98			min	-75.202	3	0	1	-14.807	4	0	1	-.013	4	0	1
99		12	max	307.114	1	0	1	.033	10	0	1	0	10	0	1
100			min	-75.153	3	0	1	-14.863	4	0	1	-.014	4	0	1
101		13	max	307.178	1	0	1	.033	10	0	1	0	10	0	1
102			min	-75.105	3	0	1	-14.919	4	0	1	-.016	4	0	1
103		14	max	307.243	1	0	1	.033	10	0	1	0	10	0	1
104			min	-75.056	3	0	1	-14.975	4	0	1	-.017	4	0	1
105		15	max	307.308	1	0	1	.033	10	0	1	0	10	0	1
106			min	-75.008	3	0	1	-15.031	4	0	1	-.018	4	0	1
107		16	max	307.372	1	0	1	.033	10	0	1	0	10	0	1
108			min	-74.959	3	0	1	-15.087	4	0	1	-.02	4	0	1
109		17	max	307.437	1	0	1	.033	10	0	1	0	10	0	1
110			min	-74.911	3	0	1	-15.143	4	0	1	-.021	4	0	1
111		18	max	307.502	1	0	1	.033	10	0	1	0	10	0	1
112			min	-74.862	3	0	1	-15.2	4	0	1	-.022	4	0	1
113		19	max	307.566	1	0	1	.033	10	0	1	0	10	0	1
114			min	-74.814	3	0	1	-15.256	4	0	1	-.024	4	0	1
115	M6	1	max	695.253	1	.632	6	.884	4	0	3	0	3	0	1
116			min	-1089.04	3	.144	15	-.292	3	0	5	0	2	0	1
117		2	max	695.359	1	.591	6	.788	4	0	3	0	4	0	15
118			min	-1088.96	3	.134	15	-.292	3	0	5	0	2	0	6
119		3	max	695.466	1	.55	6	.691	4	0	3	0	4	0	15
120			min	-1088.88	3	.125	15	-.292	3	0	5	0	2	0	6
121		4	max	695.572	1	.509	6	.595	4	0	3	0	4	0	15
122			min	-1088.801	3	.115	15	-.292	3	0	5	0	1	0	6
123		5	max	695.679	1	.467	6	.499	4	0	3	0	4	0	15
124			min	-1088.721	3	.105	15	-.292	3	0	5	0	1	0	6
125		6	max	695.785	1	.426	6	.402	4	0	3	0	4	0	15
126			min	-1088.641	3	.095	15	-.292	3	0	5	0	3	0	6
127		7	max	695.892	1	.388	2	.306	4	0	3	0	4	0	15
128			min	-1088.561	3	.086	15	-.292	3	0	5	0	3	0	6
129		8	max	695.999	1	.356	2	.209	4	0	3	0	4	0	15
130			min	-1088.481	3	.076	15	-.292	3	0	5	0	3	0	6
131		9	max	696.105	1	.324	2	.113	4	0	3	0	4	0	15
132			min	-1088.401	3	.066	15	-.292	3	0	5	0	3	0	6
133		10	max	696.212	1	.292	2	.023	14	0	3	0	4	0	15
134			min	-1088.321	3	.057	15	-.292	3	0	5	0	3	0	6
135		11	max	696.318	1	.259	2	.022	9	0	3	0	4	0	15
136			min	-1088.241	3	.047	15	-.292	3	0	5	0	3	0	6
137		12	max	696.425	1	.227	2	.022	9	0	3	0	4	0	15
138			min	-1088.161	3	.037	15	-.292	3	0	5	0	3	0	6
139		13	max	696.531	1	.195	2	.022	9	0	3	0	4	0	15
140			min	-1088.081	3	.022	12	-.292	3	0	5	0	3	0	2
141		14	max	696.638	1	.163	2	.022	9	0	3	0	4	0	15
142			min	-1088.001	3	.002	3	-.378	5	0	5	0	3	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	696.744	1	.131	2	.022	9	0	3	0	4	0	15
144			min	-1087.922	3	-.022	3	-.474	5	0	5	0	3	0	2
145		16	max	696.851	1	.099	2	.022	9	0	3	0	4	0	15
146			min	-1087.842	3	-.046	3	-.571	5	0	5	0	3	0	2
147		17	max	696.957	1	.066	2	.022	9	0	3	0	4	0	15
148			min	-1087.762	3	-.07	3	-.667	5	0	5	0	3	0	2
149		18	max	697.064	1	.034	2	.022	9	0	3	0	4	0	15
150			min	-1087.682	3	-.094	3	-.764	5	0	5	0	3	0	2
151		19	max	697.171	1	.002	2	.022	9	0	3	0	4	0	15
152			min	-1087.602	3	-.119	3	-.86	5	0	5	0	3	0	2
153	M7	1	max	318.527	2	1.806	4	.012	3	0	9	0	4	0	2
154			min	-224.386	3	.429	15	-1.377	4	0	3	0	3	0	12
155		2	max	318.459	2	1.629	4	.012	3	0	9	0	4	0	2
156			min	-224.437	3	.387	15	-1.243	4	0	3	0	3	0	12
157		3	max	318.391	2	1.451	4	.012	3	0	9	0	1	0	2
158			min	-224.488	3	.345	15	-1.11	4	0	3	0	3	0	3
159		4	max	318.323	2	1.274	4	.012	3	0	9	0	1	0	2
160			min	-224.539	3	.304	15	-.976	4	0	3	0	3	0	3
161		5	max	318.255	2	1.096	4	.012	3	0	9	0	1	0	15
162			min	-224.589	3	.262	15	-.843	4	0	3	0	5	0	6
163		6	max	318.187	2	.918	4	.012	3	0	9	0	1	0	15
164			min	-224.64	3	.22	15	-.709	4	0	3	0	5	0	6
165		7	max	318.119	2	.741	4	.012	3	0	9	0	1	0	15
166			min	-224.691	3	.178	15	-.575	4	0	3	0	5	0	6
167		8	max	318.052	2	.563	4	.012	3	0	9	0	1	0	15
168			min	-224.742	3	.137	15	-.442	4	0	3	0	5	0	6
169		9	max	317.984	2	.385	4	.012	3	0	9	0	1	0	15
170			min	-224.793	3	.095	15	-.308	4	0	3	0	5	-.001	6
171		10	max	317.916	2	.21	2	.012	3	0	9	0	1	0	15
172			min	-224.844	3	.051	12	-.175	4	0	3	0	5	-.001	6
173		11	max	317.848	2	.072	2	.012	3	0	9	0	1	0	15
174			min	-224.895	3	-.032	3	-.041	4	0	3	0	5	-.001	6
175		12	max	317.78	2	-.031	15	.095	5	0	9	0	1	0	15
176			min	-224.946	3	-.148	6	-.018	1	0	3	0	5	-.001	6
177		13	max	317.712	2	-.072	15	.228	5	0	9	0	1	0	15
178			min	-224.997	3	-.326	6	-.018	1	0	3	0	5	-.001	6
179		14	max	317.644	2	-.114	15	.362	5	0	9	0	1	0	15
180			min	-225.048	3	-.503	6	-.018	1	0	3	0	5	-.001	6
181		15	max	317.576	2	-.156	15	.495	5	0	9	0	1	0	15
182			min	-225.098	3	-.681	6	-.018	1	0	3	0	5	0	6
183		16	max	317.509	2	-.198	15	.629	5	0	9	0	1	0	15
184			min	-225.149	3	-.859	6	-.018	1	0	3	0	5	0	6
185		17	max	317.441	2	-.239	15	.763	5	0	9	0	9	0	15
186			min	-225.2	3	-1.036	6	-.018	1	0	3	0	5	0	6
187		18	max	317.373	2	-.281	15	.896	5	0	9	0	9	0	15
188			min	-225.251	3	-1.214	6	-.018	1	0	3	0	3	0	6
189		19	max	317.305	2	-.323	15	1.03	5	0	9	0	9	0	1
190			min	-225.302	3	-1.392	6	-.018	1	0	3	0	3	0	1
191	M8	1	max	913.296	2	0	1	.09	9	0	1	0	4	0	1
192			min	-271.21	3	0	1	-14.563	4	0	1	0	3	0	1
193		2	max	913.361	2	0	1	.09	9	0	1	0	9	0	1
194			min	-271.162	3	0	1	-14.619	4	0	1	-.001	4	0	1
195		3	max	913.425	2	0	1	.09	9	0	1	0	9	0	1
196			min	-271.113	3	0	1	-14.675	4	0	1	-.003	4	0	1
197		4	max	913.49	2	0	1	.09	9	0	1	0	9	0	1
198			min	-271.065	3	0	1	-14.731	4	0	1	-.004	4	0	1
199		5	max	913.555	2	0	1	.09	9	0	1	0	9	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200		min	-271.016	3	0	1	-14.787	4	0	1	-.005	4	0	1
201	6	max	913.619	2	0	1	.09	9	0	1	0	9	0	1
202		min	-270.968	3	0	1	-14.843	4	0	1	-.007	4	0	1
203	7	max	913.684	2	0	1	.09	9	0	1	0	9	0	1
204		min	-270.919	3	0	1	-14.899	4	0	1	-.008	4	0	1
205	8	max	913.749	2	0	1	.09	9	0	1	0	9	0	1
206		min	-270.871	3	0	1	-14.956	4	0	1	-.009	4	0	1
207	9	max	913.814	2	0	1	.09	9	0	1	0	9	0	1
208		min	-270.822	3	0	1	-15.012	4	0	1	-.011	4	0	1
209	10	max	913.878	2	0	1	.09	9	0	1	0	9	0	1
210		min	-270.774	3	0	1	-15.068	4	0	1	-.012	4	0	1
211	11	max	913.943	2	0	1	.09	9	0	1	0	9	0	1
212		min	-270.725	3	0	1	-15.124	4	0	1	-.013	4	0	1
213	12	max	914.008	2	0	1	.09	9	0	1	0	9	0	1
214		min	-270.677	3	0	1	-15.18	4	0	1	-.015	4	0	1
215	13	max	914.072	2	0	1	.09	9	0	1	0	9	0	1
216		min	-270.628	3	0	1	-15.236	4	0	1	-.016	4	0	1
217	14	max	914.137	2	0	1	.09	9	0	1	0	9	0	1
218		min	-270.58	3	0	1	-15.292	4	0	1	-.017	4	0	1
219	15	max	914.202	2	0	1	.09	9	0	1	0	9	0	1
220		min	-270.531	3	0	1	-15.348	4	0	1	-.019	4	0	1
221	16	max	914.266	2	0	1	.09	9	0	1	0	9	0	1
222		min	-270.483	3	0	1	-15.404	4	0	1	-.02	4	0	1
223	17	max	914.331	2	0	1	.09	9	0	1	0	9	0	1
224		min	-270.434	3	0	1	-15.46	4	0	1	-.021	4	0	1
225	18	max	914.396	2	0	1	.09	9	0	1	0	9	0	1
226		min	-270.385	3	0	1	-15.516	4	0	1	-.023	4	0	1
227	19	max	914.461	2	0	1	.09	9	0	1	0	9	0	1
228		min	-270.337	3	0	1	-15.572	4	0	1	-.024	4	0	1
229	M10	1	max	224.174	1	.674	4	.997	5	0	1	0	1	0
230		min	-302.992	3	.17	15	-.106	1	-.001	5	0	3	0	1
231	2	max	224.28	1	.633	4	.9	5	0	1	0	4	0	15
232		min	-302.912	3	.16	15	-.106	1	-.001	5	0	3	0	4
233	3	max	224.387	1	.592	4	.804	5	0	1	0	4	0	15
234		min	-302.832	3	.151	15	-.106	1	-.001	5	0	3	0	4
235	4	max	224.494	1	.551	4	.707	5	0	1	0	4	0	15
236		min	-302.752	3	.141	15	-.106	1	-.001	5	0	3	0	4
237	5	max	224.6	1	.509	4	.611	5	0	1	0	4	0	15
238		min	-302.672	3	.131	15	-.106	1	-.001	5	0	3	0	4
239	6	max	224.707	1	.468	4	.515	5	0	1	0	5	0	15
240		min	-302.592	3	.122	15	-.106	1	-.001	5	0	3	0	4
241	7	max	224.813	1	.427	4	.418	5	0	1	0	5	0	15
242		min	-302.512	3	.112	15	-.106	1	-.001	5	0	3	0	4
243	8	max	224.92	1	.386	4	.322	5	0	1	0	5	0	15
244		min	-302.432	3	.102	15	-.106	1	-.001	5	0	3	0	4
245	9	max	225.026	1	.344	4	.225	5	0	1	0	5	0	15
246		min	-302.352	3	.092	15	-.106	1	-.001	5	0	3	0	4
247	10	max	225.133	1	.303	4	.129	5	0	1	0	5	0	15
248		min	-302.273	3	.083	15	-.106	1	-.001	5	0	3	0	4
249	11	max	225.239	1	.262	4	.032	5	0	1	0	5	0	15
250		min	-302.193	3	.073	15	-.106	1	-.001	5	0	3	0	4
251	12	max	225.346	1	.221	4	.002	10	0	1	0	5	0	15
252		min	-302.113	3	.063	15	-.106	1	-.001	5	0	3	0	4
253	13	max	225.452	1	.179	4	.002	10	0	1	0	5	0	15
254		min	-302.033	3	.054	15	-.173	4	-.001	5	0	3	0	4
255	14	max	225.559	1	.138	4	.002	10	0	1	0	5	0	15
256		min	-301.953	3	.044	15	-.269	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	225.666	1	.097	4	.002	10	0	1	0	5	0	15
258		min	-301.873	3	.028	12	-.366	4	-.001	5	0	3	0	4
259	16	max	225.772	1	.055	4	.002	10	0	1	0	5	0	15
260		min	-301.793	3	.011	9	-.462	4	-.001	5	0	3	0	4
261	17	max	225.879	1	.022	5	.002	10	0	1	0	5	0	15
262		min	-301.713	3	-.016	9	-.559	4	-.001	5	0	3	0	4
263	18	max	225.985	1	.007	5	.002	10	0	1	0	5	0	15
264		min	-301.633	3	-.042	9	-.655	4	-.001	5	0	3	0	4
265	19	max	226.092	1	-.005	15	.002	10	0	1	0	5	0	15
266		min	-301.553	3	-.072	1	-.751	4	-.001	5	0	3	0	4
267	M11	1	max	100.776	2	1.792	.128	1	0	4	0	5	0	6
268		min	-88.757	3	.418	15	-1.258	5	0	10	0	1	0	15
269	2	max	100.708	2	1.615	6	.128	1	0	4	0	5	0	2
270		min	-88.808	3	.377	15	-1.124	5	0	10	0	1	0	15
271	3	max	100.64	2	1.437	6	.128	1	0	4	0	3	0	2
272		min	-88.858	3	.335	15	-.991	5	0	10	0	1	0	3
273	4	max	100.572	2	1.259	6	.128	1	0	4	0	3	0	15
274		min	-88.909	3	.293	15	-.857	5	0	10	0	1	0	4
275	5	max	100.504	2	1.082	6	.128	1	0	4	0	3	0	15
276		min	-88.96	3	.251	15	-.723	5	0	10	0	1	0	4
277	6	max	100.436	2	.904	6	.128	1	0	4	0	3	0	15
278		min	-89.011	3	.21	15	-.59	5	0	10	0	1	0	4
279	7	max	100.369	2	.726	6	.128	1	0	4	0	3	0	15
280		min	-89.062	3	.168	15	-.456	5	0	10	0	4	0	4
281	8	max	100.301	2	.549	6	.128	1	0	4	0	3	0	15
282		min	-89.113	3	.126	15	-.322	5	0	10	0	4	-.001	4
283	9	max	100.233	2	.371	6	.128	1	0	4	0	3	0	15
284		min	-89.164	3	.084	15	-.189	5	0	10	0	4	-.001	4
285	10	max	100.165	2	.193	6	.128	1	0	4	0	3	0	15
286		min	-89.215	3	.043	15	-.055	5	0	10	0	4	-.001	4
287	11	max	100.097	2	.037	2	.128	1	0	4	0	3	0	15
288		min	-89.266	3	-.015	3	-.035	3	0	10	0	4	-.001	4
289	12	max	100.029	2	-.041	15	.241	4	0	4	0	3	0	15
290		min	-89.317	3	-.162	4	-.035	3	0	10	0	4	-.001	4
291	13	max	99.961	2	-.083	15	.375	4	0	4	0	3	0	15
292		min	-89.367	3	-.34	4	-.035	3	0	10	0	4	-.001	4
293	14	max	99.894	2	-.125	15	.509	4	0	4	0	3	0	15
294		min	-89.418	3	-.518	4	-.035	3	0	10	0	5	-.001	4
295	15	max	99.826	2	-.166	15	.642	4	0	4	0	3	0	15
296		min	-89.469	3	-.695	4	-.035	3	0	10	0	5	0	4
297	16	max	99.758	2	-.208	15	.776	4	0	4	0	3	0	15
298		min	-89.52	3	-.873	4	-.035	3	0	10	0	10	0	4
299	17	max	99.69	2	-.25	15	.909	4	0	4	0	3	0	15
300		min	-89.571	3	-1.05	4	-.035	3	0	10	0	10	0	4
301	18	max	99.622	2	-.292	15	1.043	4	0	4	0	4	0	15
302		min	-89.622	3	-1.228	4	-.035	3	0	10	0	10	0	4
303	19	max	99.554	2	-.333	15	1.177	4	0	4	0	4	0	1
304		min	-89.673	3	-1.406	4	-.035	3	0	10	0	10	0	1
305	M12	1	max	306.655	1	0	.492	1	0	1	0	4	0	1
306		min	-75.228	3	0	1	-13.389	5	0	1	0	3	0	1
307	2	max	306.719	1	0	1	.492	1	0	1	0	1	0	1
308		min	-75.18	3	0	1	-13.445	5	0	1	-.001	5	0	1
309	3	max	306.784	1	0	1	.492	1	0	1	0	1	0	1
310		min	-75.131	3	0	1	-13.501	5	0	1	-.002	5	0	1
311	4	max	306.849	1	0	1	.492	1	0	1	0	1	0	1
312		min	-75.083	3	0	1	-13.557	5	0	1	-.004	5	0	1
313	5	max	306.913	1	0	1	.492	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-75.034	3	0	1	-13.613	5	0	1	-.005	5	0	1
315	6	max	306.978	1	0	1	.492	1	0	1	0	1	0	1
316		min	-74.985	3	0	1	-13.669	5	0	1	-.006	5	0	1
317	7	max	307.043	1	0	1	.492	1	0	1	0	1	0	1
318		min	-74.937	3	0	1	-13.726	5	0	1	-.007	5	0	1
319	8	max	307.108	1	0	1	.492	1	0	1	0	1	0	1
320		min	-74.888	3	0	1	-13.782	5	0	1	-.008	5	0	1
321	9	max	307.172	1	0	1	.492	1	0	1	0	1	0	1
322		min	-74.84	3	0	1	-13.838	5	0	1	-.01	5	0	1
323	10	max	307.237	1	0	1	.492	1	0	1	0	1	0	1
324		min	-74.791	3	0	1	-13.894	5	0	1	-.011	5	0	1
325	11	max	307.302	1	0	1	.492	1	0	1	0	1	0	1
326		min	-74.743	3	0	1	-13.95	5	0	1	-.012	5	0	1
327	12	max	307.366	1	0	1	.492	1	0	1	0	1	0	1
328		min	-74.694	3	0	1	-14.006	5	0	1	-.013	5	0	1
329	13	max	307.431	1	0	1	.492	1	0	1	0	1	0	1
330		min	-74.646	3	0	1	-14.062	5	0	1	-.015	5	0	1
331	14	max	307.496	1	0	1	.492	1	0	1	0	1	0	1
332		min	-74.597	3	0	1	-14.118	5	0	1	-.016	5	0	1
333	15	max	307.561	1	0	1	.492	1	0	1	0	1	0	1
334		min	-74.549	3	0	1	-14.174	5	0	1	-.017	5	0	1
335	16	max	307.625	1	0	1	.492	1	0	1	0	1	0	1
336		min	-74.5	3	0	1	-14.23	5	0	1	-.019	5	0	1
337	17	max	307.69	1	0	1	.492	1	0	1	0	1	0	1
338		min	-74.452	3	0	1	-14.286	5	0	1	-.02	5	0	1
339	18	max	307.755	1	0	1	.492	1	0	1	0	1	0	1
340		min	-74.403	3	0	1	-14.342	5	0	1	-.021	5	0	1
341	19	max	307.819	1	0	1	.492	1	0	1	0	1	0	1
342		min	-74.355	3	0	1	-14.399	5	0	1	-.022	5	0	1
343	M1	1	max	58.549	1	337.118	3	.703	10	0	.026	4	0	2
344		min	3.232	10	-226.986	1	-14.772	4	0	3	-.001	10	0	3
345	2	max	58.644	1	336.921	3	.703	10	0	2	.023	4	.05	1
346		min	3.311	10	-227.248	1	-14.53	4	0	3	-.001	10	-.073	3
347	3	max	46.755	1	4.542	4	.7	10	0	5	.019	4	.098	1
348		min	-2.118	10	-19.06	3	-13.383	4	0	1	-.001	10	-.145	3
349	4	max	46.85	1	4.205	4	.7	10	0	5	.016	4	.102	2
350		min	-2.038	10	-19.256	3	-13.141	4	0	1	0	10	-.141	3
351	5	max	46.946	1	3.874	14	.7	10	0	5	.014	4	.105	2
352		min	-1.958	10	-19.453	3	-12.899	4	0	1	0	10	-.137	3
353	6	max	47.041	1	3.616	14	.7	10	0	5	.011	1	.109	2
354		min	-1.879	10	-19.65	3	-12.657	4	0	1	0	10	-.132	3
355	7	max	47.137	1	3.358	14	.7	10	0	5	.008	1	.113	2
356		min	-1.799	10	-19.847	3	-12.415	4	0	1	0	10	-.128	3
357	8	max	47.232	1	3.101	14	.7	10	0	5	.005	1	.117	2
358		min	-1.72	10	-20.044	3	-12.402	1	0	1	0	10	-.124	3
359	9	max	47.328	1	2.843	14	.7	10	0	5	.003	4	.121	2
360		min	-1.64	10	-20.24	3	-12.402	1	0	1	0	10	-.119	3
361	10	max	47.423	1	2.585	14	.7	10	0	5	.002	3	.125	2
362		min	-1.56	10	-20.437	3	-12.402	1	0	1	0	10	-.115	3
363	11	max	47.519	1	2.327	14	.7	10	0	5	0	3	.13	2
364		min	-1.481	10	-20.634	3	-12.402	1	0	1	-.003	1	-.111	3
365	12	max	47.614	1	2.069	14	.7	10	0	5	0	10	.134	2
366		min	-1.401	10	-20.831	3	-12.402	1	0	1	-.005	1	-.106	3
367	13	max	47.71	1	1.812	14	.7	10	0	5	0	10	.138	2
368		min	-1.322	10	-21.028	3	-12.402	1	0	1	-.008	1	-.102	3
369	14	max	47.805	1	1.578	9	.7	10	0	5	0	10	.142	2
370		min	-1.242	10	-21.224	3	-12.402	1	0	1	-.011	1	-.097	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	47.901	1	1.36	9	.7	10	0	5	0	10	.147	2
372		min	-1.162	10	-21.421	3	-12.402	1	0	1	-.013	1	-.092	3
373	16	max	79.838	2	50.887	2	.707	10	0	1	0	10	.151	2
374		min	-31.045	3	-86.231	3	-12.517	1	0	5	-.016	1	-.087	3
375	17	max	79.933	2	50.625	2	.707	10	0	1	.001	10	.14	2
376		min	-30.973	3	-86.427	3	-12.517	1	0	5	-.019	1	-.068	3
377	18	max	-3.225	12	323.685	2	.74	10	0	3	.001	10	.071	2
378		min	-58.611	1	-157.817	3	-22.756	4	0	2	-.022	1	-.034	3
379	19	max	-3.177	12	323.423	2	.74	10	0	3	.001	10	0	2
380		min	-58.515	1	-158.013	3	-22.514	4	0	2	-.026	4	0	3
381	M5	1	max	147.967	1	1073.036	3	0	11	0	.028	4	0	3
382		min	-3.912	3	-717.293	1	-64.612	3	0	3	0	11	0	2
383	2	max	148.062	1	1072.839	3	0	11	0	9	.024	4	.155	1
384		min	-3.84	3	-717.555	1	-64.612	3	0	3	-.004	3	-.232	3
385	3	max	109.936	1	5.561	9	6.892	3	0	3	.02	4	.308	1
386		min	-.438	15	-68.529	3	-15.828	4	0	4	-.018	3	-.46	3
387	4	max	110.031	1	5.342	9	6.892	3	0	3	.017	4	.319	2
388		min	-.409	15	-68.726	3	-15.586	4	0	4	-.016	3	-.445	3
389	5	max	110.127	1	5.124	9	6.892	3	0	3	.013	4	.332	2
390		min	-.38	15	-68.922	3	-15.344	4	0	4	-.015	3	-.43	3
391	6	max	110.222	1	4.905	9	6.892	3	0	3	.01	4	.345	2
392		min	-.351	15	-69.119	3	-15.102	4	0	4	-.013	3	-.415	3
393	7	max	110.318	1	4.686	9	6.892	3	0	3	.007	4	.358	2
394		min	-.323	15	-69.316	3	-14.86	4	0	4	-.012	3	-.4	3
395	8	max	110.413	1	4.468	9	6.892	3	0	3	.004	4	.371	2
396		min	-.294	15	-69.513	3	-14.618	4	0	4	-.01	3	-.385	3
397	9	max	110.509	1	4.249	9	6.892	3	0	3	0	4	.384	2
398		min	-.265	15	-69.71	3	-14.376	4	0	4	-.009	3	-.37	3
399	10	max	110.604	1	4.03	9	6.892	3	0	3	0	1	.397	2
400		min	-.236	15	-69.906	3	-14.134	4	0	4	-.007	3	-.355	3
401	11	max	110.7	1	3.812	9	6.892	3	0	3	0	2	.411	2
402		min	-.207	15	-70.103	3	-13.892	4	0	4	-.006	3	-.339	3
403	12	max	110.795	1	3.593	9	6.892	3	0	3	0	2	.424	2
404		min	-.178	15	-70.3	3	-13.65	4	0	4	-.009	4	-.324	3
405	13	max	110.891	1	3.374	9	6.892	3	0	3	0	2	.437	2
406		min	-.15	15	-70.497	3	-13.408	4	0	4	-.011	4	-.309	3
407	14	max	110.986	1	3.156	9	6.892	3	0	3	0	11	.451	2
408		min	-.121	15	-70.694	3	-13.166	4	0	4	-.014	4	-.294	3
409	15	max	111.082	1	2.937	9	6.892	3	0	3	0	3	.464	2
410		min	-.092	15	-70.89	3	-12.924	4	0	4	-.017	4	-.278	3
411	16	max	249.517	2	168.037	2	6.863	3	0	3	.001	3	.476	2
412		min	-94.447	3	-233.341	3	-11.658	4	0	4	-.02	4	-.261	3
413	17	max	249.612	2	167.775	2	6.863	3	0	3	.003	3	.439	2
414		min	-94.376	3	-233.537	3	-11.416	4	0	4	-.023	4	-.211	3
415	18	max	-1.537	12	1022.108	2	6.347	3	0	4	.004	3	.221	2
416		min	-148.142	1	-489.36	3	-25.463	5	0	9	-.028	4	-.106	3
417	19	max	-1.489	12	1021.846	2	6.347	3	0	4	.006	3	0	3
418		min	-148.047	1	-489.557	3	-25.221	5	0	9	-.034	4	0	2
419	M9	1	max	58.538	1	337.049	3	106.708	4	0	.001	10	0	2
420		min	-.056	15	-226.986	1	-.703	10	0	2	-.025	1	0	3
421	2	max	58.634	1	336.852	3	106.95	4	0	3	.023	5	.05	1
422		min	-.027	15	-227.248	1	-.703	10	0	2	-.022	1	-.073	3
423	3	max	47.066	1	3.973	9	12.348	1	0	1	.044	5	.098	2
424		min	-1.815	10	-18.961	3	-20.527	5	0	10	-.019	1	-.145	3
425	4	max	47.161	1	3.754	9	12.348	1	0	1	.039	5	.102	2
426		min	-1.736	10	-19.158	3	-20.285	5	0	10	-.016	1	-.141	3
427	5	max	47.257	1	3.535	9	12.348	1	0	1	.035	5	.105	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-1.656	10	-19.354	3	-20.043	5	0	10	-.013	1	-.137	3
429	6	max	47.353	1	3.317	9	12.348	1	0	1	.031	5	-.109	2
430		min	-1.577	10	-19.551	3	-19.801	5	0	10	-.011	1	-.132	3
431	7	max	47.448	1	3.098	9	12.348	1	0	1	.026	5	.113	2
432		min	-1.497	10	-19.748	3	-19.559	5	0	10	-.008	1	-.128	3
433	8	max	47.544	1	2.879	9	12.348	1	0	1	.022	5	.117	2
434		min	-1.417	10	-19.945	3	-19.317	5	0	10	-.005	1	-.124	3
435	9	max	47.639	1	2.661	9	12.348	1	0	1	.018	5	.121	2
436		min	-1.338	10	-20.142	3	-19.075	5	0	10	-.003	1	-.119	3
437	10	max	47.735	1	2.442	9	12.348	1	0	1	.014	4	.125	2
438		min	-1.258	10	-20.338	3	-18.833	5	0	10	0	1	-.115	3
439	11	max	47.83	1	2.223	9	12.348	1	0	1	.01	4	.13	2
440		min	-1.179	10	-20.535	3	-18.591	5	0	10	0	10	-.111	3
441	12	max	47.926	1	2.005	9	12.348	1	0	1	.008	3	.134	2
442		min	-1.099	10	-20.732	3	-18.349	5	0	10	0	10	-.106	3
443	13	max	48.021	1	1.786	9	12.348	1	0	1	.008	1	.138	2
444		min	-1.019	10	-20.929	3	-18.107	5	0	10	0	10	-.102	3
445	14	max	48.117	1	1.567	9	12.348	1	0	1	.011	1	.142	2
446		min	-.94	10	-21.126	3	-17.865	5	0	10	-.002	5	-.097	3
447	15	max	48.212	1	1.349	9	12.348	1	0	1	.013	1	.147	2
448		min	-.86	10	-21.322	3	-17.623	5	0	10	-.006	5	-.092	3
449	16	max	79.932	2	50.629	2	12.466	1	0	10	.016	1	.151	2
450		min	-31.999	3	-86.655	3	-16.235	5	0	4	-.009	5	-.087	3
451	17	max	80.027	2	50.367	2	12.466	1	0	10	.019	1	.14	2
452		min	-31.928	3	-86.851	3	-15.993	5	0	4	-.012	5	-.068	3
453	18	max	8.825	5	323.685	2	12.956	1	0	2	.022	1	.071	2
454		min	-58.588	1	-157.809	3	-29.136	5	0	3	-.019	5	-.034	3
455	19	max	8.87	5	323.423	2	12.956	1	0	2	.024	1	0	2
456		min	-58.492	1	-158.005	3	-28.894	5	0	3	-.025	5	0	3
457	M13	1	max	106.707	4	226.83	1	.056	15	0	.025	1	0	2
458		min	-.703	10	-337.09	3	-58.536	1	0	3	-.001	10	0	3
459	2	max	102.615	4	161.853	1	.718	5	0	2	.012	3	.12	3
460		min	-.703	10	-240.024	3	-43.81	1	0	3	-.003	2	-.081	1
461	3	max	98.523	4	97.037	2	1.472	5	0	2	.01	3	.2	3
462		min	-.703	10	-142.958	3	-29.085	1	0	3	-.012	1	-.135	1
463	4	max	94.43	4	32.304	2	2.225	5	0	2	.007	3	.239	3
464		min	-.703	10	-45.893	3	-14.36	1	0	3	-.021	1	-.162	2
465	5	max	90.338	4	51.173	3	3.228	2	0	2	.005	3	.238	3
466		min	-.703	10	-33.079	1	-5.113	3	0	3	-.024	1	-.162	2
467	6	max	86.246	4	148.239	3	15.091	1	0	2	.004	5	.197	3
468		min	-.703	10	-98.056	1	-4.404	3	0	3	-.021	1	-.135	2
469	7	max	82.153	4	245.305	3	29.816	1	0	2	.005	5	.115	3
470		min	-.703	10	-163.033	1	-3.695	3	0	3	-.011	1	-.081	2
471	8	max	78.061	4	342.371	3	44.541	1	0	2	.008	4	.002	1
472		min	-.703	10	-228.01	1	-2.986	3	0	3	0	3	-.008	3
473	9	max	73.969	4	439.437	3	59.267	1	0	2	.026	1	.11	1
474		min	-.703	10	-292.987	1	-2.278	3	0	3	-.001	3	-.171	3
475	10	max	69.876	4	-7.434	15	73.992	1	0	2	.054	1	.246	1
476		min	-.703	10	-536.503	3	1.187	12	0	3	-.016	5	-.374	3
477	11	max	47.495	4	292.987	1	7.015	5	0	3	.026	1	.11	1
478		min	-.703	10	-439.437	3	-59.256	1	0	2	-.013	5	-.171	3
479	12	max	43.402	4	228.01	1	7.769	5	0	3	.005	2	.002	1
480		min	-.703	10	-342.371	3	-44.531	1	0	2	-.01	5	-.008	3
481	13	max	39.31	4	163.033	1	8.523	5	0	3	0	10	.115	3
482		min	-.703	10	-245.305	3	-29.805	1	0	2	-.011	1	-.081	2
483	14	max	35.218	4	98.056	1	9.276	5	0	3	-.001	10	.197	3
484		min	-.703	10	-148.239	3	-15.08	1	0	2	-.021	1	-.135	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	31.125	4	33.079	1	10.418	4	0	3	.001	5	.238	3
486		min	-.703	10	-51.173	3	-3.228	2	0	2	-.024	1	-.162	2
487	16	max	27.033	4	45.893	3	14.37	1	0	3	.006	5	.239	3
488		min	-.703	10	-32.304	2	-.806	10	0	2	-.021	1	-.162	2
489	17	max	22.941	4	142.959	3	29.096	1	0	3	.01	5	.2	3
490		min	-.703	10	-97.037	2	.54	10	0	2	-.012	1	-.135	1
491	18	max	18.848	4	240.024	3	43.821	1	0	3	.016	4	.12	3
492		min	-.703	10	-161.853	1	1.886	10	0	2	-.003	2	-.081	1
493	19	max	14.756	4	337.09	3	58.546	1	0	3	.026	4	0	2
494		min	-.703	10	-226.83	1	3.232	10	0	2	-.001	10	0	3
495	M16	1	max	28.883	5	323.495	2	8.87	5	0	.024	1	0	2
496		min	-12.938	1	-158.024	3	-58.495	1	0	2	-.025	5	0	3
497	2	max	24.791	5	230.739	2	9.623	5	0	3	.004	9	.057	3
498		min	-12.938	1	-113.267	3	-43.77	1	0	2	-.021	5	-.115	2
499	3	max	20.699	5	137.983	2	10.377	5	0	3	0	3	.094	3
500		min	-12.938	1	-68.511	3	-29.045	1	0	2	-.019	4	-.192	2
501	4	max	16.606	5	45.227	2	11.131	5	0	3	-.001	12	.114	3
502		min	-12.938	1	-23.754	3	-14.32	1	0	2	-.021	1	-.23	2
503	5	max	12.514	5	21.003	3	11.884	5	0	3	-.002	12	.114	3
504		min	-12.938	1	-47.529	2	-3.243	3	0	2	-.024	1	-.23	2
505	6	max	8.422	5	65.76	3	15.353	4	0	3	-.001	10	.096	3
506		min	-12.938	1	-140.285	2	-2.534	3	0	2	-.021	1	-.191	2
507	7	max	4.329	5	110.516	3	29.856	1	0	3	.003	5	.059	3
508		min	-12.938	1	-233.04	2	-1.825	3	0	2	-.011	1	-.113	2
509	8	max	2.37	3	155.273	3	44.581	1	0	3	.009	4	.004	3
510		min	-12.938	1	-325.796	2	-1.116	3	0	2	-.006	3	0	5
511	9	max	2.37	3	200.03	3	59.307	1	0	3	.026	1	.158	2
512		min	-12.938	1	-418.552	2	-.407	3	0	2	-.006	3	-.07	3
513	10	max	17.279	5	-7.358	15	74.032	1	0	14	.054	1	.352	2
514		min	-12.938	1	-511.308	2	-1.416	3	0	2	-.006	3	-.163	3
515	11	max	13.187	5	418.552	2	5.94	5	0	2	.026	1	.158	2
516		min	-12.924	1	-200.03	3	-59.284	1	0	3	-.01	5	-.07	3
517	12	max	9.094	5	325.796	2	6.694	5	0	2	.005	2	.004	3
518		min	-12.924	1	-155.273	3	-44.558	1	0	3	-.007	5	0	15
519	13	max	5.002	5	233.04	2	7.447	5	0	2	0	10	.059	3
520		min	-12.924	1	-110.516	3	-29.833	1	0	3	-.011	1	-.113	2
521	14	max	.91	5	140.285	2	8.201	5	0	2	0	12	.096	3
522		min	-12.924	1	-65.76	3	-15.108	1	0	3	-.021	1	-.191	2
523	15	max	.74	10	47.529	2	9.32	4	0	2	.003	5	.114	3
524		min	-12.924	1	-21.003	3	-3.251	2	0	3	-.024	1	-.23	2
525	16	max	.74	10	23.754	3	14.343	1	0	2	.006	5	.114	3
526		min	-12.924	1	-45.227	2	-.817	10	0	3	-.021	1	-.23	2
527	17	max	.74	10	68.511	3	29.068	1	0	2	.011	5	.094	3
528		min	-14.347	4	-137.983	2	.529	10	0	3	-.012	1	-.192	2
529	18	max	.74	10	113.267	3	43.793	1	0	2	.016	4	.057	3
530		min	-18.439	4	-230.739	2	1.875	10	0	3	-.003	2	-.115	2
531	19	max	.74	10	158.024	3	58.518	1	0	2	.026	4	0	2
532		min	-22.532	4	-323.495	2	3.177	12	0	3	-.001	10	0	3
533	M15	1	max	0	.776	3	.152	3	0	1	0	1	0	1
534		min	-88.892	3	0	1	0	1	0	3	0	3	0	1
535	2	max	0	1	.69	3	.152	3	0	1	0	1	0	1
536		min	-88.952	3	0	1	0	1	0	3	0	3	0	3
537	3	max	0	1	.604	3	.152	3	0	1	0	1	0	1
538		min	-89.012	3	0	1	0	1	0	3	0	3	0	3
539	4	max	0	1	.518	3	.152	3	0	1	0	1	0	1
540		min	-89.071	3	0	1	0	1	0	3	0	3	0	3
541	5	max	0	1	.431	3	.152	3	0	1	0	1	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-89.131	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.345	3	.152	3	0	1	0	1	0	1
544			min	-89.191	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.259	3	.152	3	0	1	0	3	0	1
546			min	-89.25	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.173	3	.152	3	0	1	0	3	0	1
548			min	-89.31	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.086	3	.152	3	0	1	0	3	0	1
550			min	-89.37	3	0	1	0	1	0	3	0	1	0	3
551		10	max	0	1	0	1	.152	3	0	1	0	3	0	1
552			min	-89.429	3	0	1	0	1	0	3	0	1	0	3
553		11	max	0	1	0	1	.152	3	0	1	0	3	0	1
554			min	-89.489	3	-.086	3	0	1	0	3	0	1	0	3
555		12	max	0	1	0	1	.152	3	0	1	0	3	0	1
556			min	-89.549	3	-.173	3	0	1	0	3	0	1	0	3
557		13	max	0	1	0	1	.152	3	0	1	0	3	0	1
558			min	-89.608	3	-.259	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.152	3	0	1	0	3	0	1
560			min	-89.668	3	-.345	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.152	3	0	1	0	3	0	1
562			min	-89.728	3	-.431	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.152	3	0	1	0	3	0	1
564			min	-89.787	3	-.518	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.152	3	0	1	0	3	0	1
566			min	-89.847	3	-.604	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.152	3	0	1	0	3	0	1
568			min	-89.907	3	-.69	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.152	3	0	1	0	3	0	1
570			min	-89.966	3	-.776	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	1.936	4	.264	4	0	3	0	3	0	1
572			min	-158.044	4	0	2	-.061	3	0	1	0	4	0	1
573		2	max	0	2	1.72	4	.239	4	0	3	0	3	0	2
574			min	-158.039	4	0	2	-.061	3	0	1	0	4	0	4
575		3	max	0	2	1.505	4	.214	4	0	3	0	3	0	2
576			min	-158.035	4	0	2	-.061	3	0	1	0	4	0	4
577		4	max	0	2	1.29	4	.188	4	0	3	0	3	0	2
578			min	-158.03	4	0	2	-.061	3	0	1	0	1	-.001	4
579		5	max	0	2	1.075	4	.163	4	0	3	0	3	0	2
580			min	-158.025	4	0	2	-.061	3	0	1	0	1	-.002	4
581		6	max	0	2	.86	4	.138	4	0	3	0	3	0	2
582			min	-158.021	4	0	2	-.061	3	0	1	0	1	-.002	4
583		7	max	0	2	.645	4	.113	4	0	3	0	3	0	2
584			min	-158.016	4	0	2	-.061	3	0	1	0	1	-.002	4
585		8	max	0	2	.43	4	.087	4	0	3	0	5	0	2
586			min	-158.012	4	0	2	-.061	3	0	1	0	1	-.002	4
587		9	max	0	2	.215	4	.062	4	0	3	0	5	0	2
588			min	-158.007	4	0	2	-.061	3	0	1	0	1	-.002	4
589		10	max	0	2	0	1	.047	1	0	3	0	5	0	2
590			min	-158.002	4	0	1	-.061	3	0	1	0	1	-.002	4
591		11	max	0	2	0	2	.047	1	0	3	0	5	0	2
592			min	-157.998	4	-.215	4	-.061	3	0	1	0	1	-.002	4
593		12	max	.077	1	0	2	.047	1	0	3	0	5	0	2
594			min	-157.993	4	-.43	4	-.061	3	0	1	0	1	-.002	4
595		13	max	.156	1	0	2	.047	1	0	3	0	5	0	2
596			min	-157.989	4	-.645	4	-.061	3	0	1	0	3	-.002	4
597		14	max	.236	1	0	2	.047	1	0	3	0	5	0	2
598			min	-157.984	4	-.86	4	-.069	5	0	1	0	3	-.002	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.315	1	0	2	.047	1	0	3	0	5	0	2
600		min	-157.979	4	-1.075	4	-.094	5	0	1	0	3	-.002	4
601	16	max	.395	1	0	2	.047	1	0	3	0	4	0	2
602		min	-157.975	4	-1.29	4	-.119	5	0	1	0	3	-.001	4
603	17	max	.474	1	0	2	.047	1	0	3	0	1	0	2
604		min	-158.009	5	-1.505	4	-.145	5	0	1	0	3	0	4
605	18	max	.554	1	0	2	.047	1	0	3	0	1	0	2
606		min	-158.07	5	-1.72	4	-.17	5	0	1	0	3	0	4
607	19	max	.634	1	0	2	.047	1	0	3	0	1	0	1
608		min	-158.13	5	-1.936	4	-.195	5	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.007	2	.002	1	8.143e-4	5	NC	3	NC	1	
2			min	-.003	3	-.006	3	-.008	5	-1.906e-4	1	4998.868	2	NC	1	
3			2	max	.002	1	.006	2	.002	1	8.337e-4	5	NC	3	NC	1
4				min	-.003	3	-.006	3	-.008	5	-1.82e-4	1	5439.83	2	NC	1
5			3	max	.002	1	.006	2	.001	1	8.531e-4	5	NC	1	NC	1
6				min	-.003	3	-.006	3	-.008	5	-1.735e-4	1	5961.642	2	NC	1
7			4	max	.002	1	.005	2	.001	1	8.724e-4	5	NC	1	NC	1
8				min	-.002	3	-.005	3	-.007	5	-1.649e-4	1	6583.583	2	NC	1
9			5	max	.001	1	.005	2	.001	1	8.918e-4	5	NC	1	NC	1
10				min	-.002	3	-.005	3	-.007	5	-1.564e-4	1	7331.281	2	NC	1
11			6	max	.001	1	.004	2	.001	1	9.112e-4	5	NC	1	NC	1
12				min	-.002	3	-.005	3	-.007	5	-1.478e-4	1	8239.384	2	NC	1
13			7	max	.001	1	.004	2	0	1	9.306e-4	5	NC	1	NC	1
14				min	-.002	3	-.005	3	-.006	5	-1.393e-4	1	9355.649	2	NC	1
15			8	max	.001	1	.003	2	0	1	9.5e-4	5	NC	1	NC	1
16				min	-.002	3	-.004	3	-.006	5	-1.307e-4	1	NC	1	NC	1
17			9	max	.001	1	.003	2	0	1	9.694e-4	5	NC	1	NC	1
18				min	-.002	3	-.004	3	-.006	5	-1.222e-4	1	NC	1	NC	1
19			10	max	0	1	.002	2	0	1	9.887e-4	5	NC	1	NC	1
20				min	-.001	3	-.004	3	-.005	5	-1.136e-4	1	NC	1	NC	1
21		11	max	0	1	.002	2	0	1	1.008e-3	5	NC	1	NC	1	
22			min	-.001	3	-.003	3	-.005	5	-1.051e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	0	1	1.028e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.004	5	-9.656e-5	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	1.047e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.004	5	-8.801e-5	1	NC	1	NC	1	
27		14	max	0	1	0	2	0	1	1.066e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.003	5	-7.946e-5	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.086e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.003	5	-7.092e-5	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.105e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.002	5	-6.237e-5	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.124e-3	5	NC	1	NC	1	
34			min	0	3	0	3	-.001	5	-5.383e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.144e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-4.528e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.163e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.673e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.696e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.346e-4	5	NC	1	NC	1	
41		2	max	0	3	0	2	.003	5	2.405e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-5.375e-4	5	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	3.115e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	9	-5.404e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.008	5	3.824e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	9	-5.432e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.011	4	4.534e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.461e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.014	4	5.243e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.489e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.017	4	5.953e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-5.518e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.019	4	6.662e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.547e-4	5	NC	1	NC	1
55		9	max	0	3	0	2	.022	4	7.372e-5	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-5.575e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.025	4	8.081e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.604e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.027	4	8.79e-5	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-5.633e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.029	4	9.5e-5	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-5.661e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.032	4	1.021e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-5.69e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.034	4	1.092e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-5.719e-4	5	NC	1	NC	1
67		15	max	0	3	.004	2	.036	4	1.163e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-5.747e-4	5	NC	1	NC	1
69		16	max	0	3	.005	2	.038	4	1.234e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-5.776e-4	5	8831.148	2	NC	1
71		17	max	0	3	.006	2	.04	4	1.305e-4	1	NC	1	NC	1
72			min	-.001	2	-.007	3	0	10	-5.805e-4	5	7526.36	2	NC	1
73		18	max	0	3	.007	2	.042	4	1.376e-4	1	NC	3	NC	1
74			min	-.001	2	-.007	3	0	10	-5.833e-4	5	6520.266	2	NC	1
75		19	max	.001	3	.008	2	.044	4	1.447e-4	1	NC	3	NC	1
76			min	-.001	2	-.007	3	0	10	-5.862e-4	5	5736.088	2	NC	1
77	M4	1	max	.001	1	.008	2	0	10	2.193e-3	5	NC	1	NC	1
78			min	0	3	-.006	3	-.046	4	-1.521e-4	1	NC	1	418.738	4
79		2	max	.001	1	.007	2	0	10	2.193e-3	5	NC	1	NC	1
80			min	0	3	-.006	3	-.042	4	-1.521e-4	1	NC	1	456.42	4
81		3	max	.001	1	.007	2	0	10	2.193e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.039	4	-1.521e-4	1	NC	1	501.262	4
83		4	max	.001	1	.006	2	0	10	2.193e-3	5	NC	1	NC	1
84			min	0	3	-.005	3	-.035	4	-1.521e-4	1	NC	1	555.149	4
85		5	max	.001	1	.006	2	0	10	2.193e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.031	4	-1.521e-4	1	NC	1	620.649	4
87		6	max	.001	1	.005	2	0	10	2.193e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.028	4	-1.521e-4	1	NC	1	701.333	4
89		7	max	0	1	.005	2	0	10	2.193e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.024	4	-1.521e-4	1	NC	1	802.288	4
91		8	max	0	1	.005	2	0	10	2.193e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.021	4	-1.521e-4	1	NC	1	930.96	4
93		9	max	0	1	.004	2	0	10	2.193e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.018	4	-1.521e-4	1	NC	1	1098.612	4
95		10	max	0	1	.004	2	0	10	2.193e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.015	4	-1.521e-4	1	NC	1	1322.946	4
97		11	max	0	1	.003	2	0	10	2.193e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.012	4	-1.521e-4	1	NC	1	1633.118	4
99		12	max	0	1	.003	2	0	10	2.193e-3	5	NC	1	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	3	-.002	3	-.009	4	-1.521e-4	1	NC	1	2079.977	4
101		13	max	0	1	.003	2	0	10	2.193e-3	5	NC	1	NC	1
102			min	0	3	-.002	3	-.007	4	-1.521e-4	1	NC	1	2758.899	4
103		14	max	0	1	.002	2	0	10	2.193e-3	5	NC	1	NC	1
104			min	0	3	-.002	3	-.005	4	-1.521e-4	1	NC	1	3866.601	4
105		15	max	0	1	.002	2	0	10	2.193e-3	5	NC	1	NC	1
106			min	0	3	-.001	3	-.003	4	-1.521e-4	1	NC	1	5865.205	4
107		16	max	0	1	.001	2	0	10	2.193e-3	5	NC	1	NC	1
108			min	0	3	-.001	3	-.002	4	-1.521e-4	1	NC	1	NC	1
109		17	max	0	1	0	2	0	10	2.193e-3	5	NC	1	NC	1
110			min	0	3	0	3	0	4	-1.521e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	2.193e-3	5	NC	1	NC	1
112			min	0	3	0	3	0	4	-1.521e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	2.193e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.521e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.021	2	0	9	8.608e-4	4	NC	3	NC	1
116			min	-.009	3	-.018	3	-.008	5	-8.791e-8	2	1577.727	2	6724.388	3
117		2	max	.005	1	.02	2	0	9	8.803e-4	4	NC	3	NC	1
118			min	-.008	3	-.017	3	-.008	5	-9.681e-7	1	1686.725	2	7181.237	3
119		3	max	.005	1	.018	2	0	9	8.997e-4	4	NC	3	NC	1
120			min	-.008	3	-.016	3	-.008	5	-2.195e-6	1	1811.449	2	7718.964	3
121		4	max	.005	1	.017	2	0	9	9.191e-4	4	NC	3	NC	1
122			min	-.007	3	-.015	3	-.007	5	-3.423e-6	1	1955.065	2	8355.493	3
123		5	max	.004	1	.016	2	0	9	9.385e-4	4	NC	3	NC	1
124			min	-.007	3	-.014	3	-.007	5	-4.65e-6	1	2121.655	2	9114.503	3
125		6	max	.004	1	.014	2	0	9	9.579e-4	4	NC	3	NC	1
126			min	-.006	3	-.013	3	-.007	5	-5.877e-6	1	2316.563	2	NC	1
127		7	max	.004	1	.013	2	0	9	9.773e-4	4	NC	3	NC	1
128			min	-.006	3	-.012	3	-.007	5	-7.104e-6	1	2546.926	2	NC	1
129		8	max	.003	1	.012	2	0	9	9.968e-4	4	NC	3	NC	1
130			min	-.005	3	-.011	3	-.006	5	-8.331e-6	1	2822.487	2	NC	1
131		9	max	.003	1	.011	2	0	9	1.016e-3	4	NC	3	NC	1
132			min	-.005	3	-.01	3	-.006	5	-9.559e-6	1	3156.893	2	NC	1
133		10	max	.003	1	.009	2	0	9	1.036e-3	4	NC	3	NC	1
134			min	-.004	3	-.009	3	-.005	5	-1.079e-5	1	3569.869	2	NC	1
135		11	max	.003	1	.008	2	0	9	1.055e-3	4	NC	3	NC	1
136			min	-.004	3	-.008	3	-.005	5	-1.201e-5	1	4091.01	2	NC	1
137		12	max	.002	1	.007	2	0	9	1.074e-3	4	NC	3	NC	1
138			min	-.003	3	-.007	3	-.004	5	-1.324e-5	1	4766.834	2	NC	1
139		13	max	.002	1	.006	2	0	9	1.094e-3	4	NC	3	NC	1
140			min	-.003	3	-.006	3	-.004	5	-1.447e-5	1	5674.88	2	NC	1
141		14	max	.002	1	.005	2	0	9	1.113e-3	4	NC	1	NC	1
142			min	-.002	3	-.005	3	-.003	5	-1.569e-5	1	6954.725	2	NC	1
143		15	max	.001	1	.004	2	0	9	1.133e-3	4	NC	1	NC	1
144			min	-.002	3	-.004	3	-.003	5	-1.692e-5	1	8885.526	2	NC	1
145		16	max	0	1	.003	2	0	9	1.152e-3	4	NC	1	NC	1
146			min	-.001	3	-.003	3	-.002	5	-1.815e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	9	1.172e-3	4	NC	1	NC	1
148			min	0	3	-.002	3	-.001	5	-1.938e-5	1	NC	1	NC	1
149		18	max	0	1	0	2	0	1	1.191e-3	4	NC	1	NC	1
150			min	0	3	-.001	3	0	5	-2.06e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.21e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-2.183e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.006e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-5.562e-4	4	NC	1	NC	1
155		2	max	0	3	.001	2	.003	4	9.814e-6	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-5.484e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	9.568e-6	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-5.406e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.009	4	9.321e-6	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-5.329e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.012	4	9.075e-6	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-5.251e-4	4	NC	1	NC	1
163		6	max	0	3	.005	2	.015	4	1.283e-5	3	NC	1	NC	1
164			min	-.001	2	-.008	3	0	1	-5.173e-4	4	8462.175	2	NC	1
165		7	max	0	3	.007	2	.018	4	3.338e-5	3	NC	1	NC	1
166			min	-.001	2	-.009	3	0	1	-5.095e-4	4	7016.788	2	NC	1
167		8	max	0	3	.008	2	.02	4	5.393e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-5.018e-4	4	5952.111	2	NC	1
169		9	max	.001	3	.009	2	.023	4	7.448e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-4.94e-4	4	5130.539	2	NC	1
171		10	max	.001	3	.01	2	.026	4	9.503e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	9	-4.862e-4	4	4475.817	2	NC	1
173		11	max	.001	3	.012	2	.028	4	1.156e-4	3	NC	3	NC	1
174			min	-.002	2	-.014	3	0	9	-4.784e-4	4	3942.07	2	NC	1
175		12	max	.002	3	.013	2	.031	4	1.361e-4	3	NC	3	NC	1
176			min	-.002	2	-.015	3	0	9	-4.706e-4	4	3499.801	2	NC	1
177		13	max	.002	3	.015	2	.033	4	1.567e-4	3	NC	3	NC	1
178			min	-.002	2	-.016	3	0	9	-4.629e-4	4	3128.967	2	NC	1
179		14	max	.002	3	.016	2	.035	4	1.772e-4	3	NC	3	NC	1
180			min	-.003	2	-.017	3	0	9	-4.551e-4	4	2815.305	2	NC	1
181		15	max	.002	3	.018	2	.037	4	1.978e-4	3	NC	3	NC	1
182			min	-.003	2	-.018	3	0	9	-4.473e-4	4	2548.272	2	NC	1
183		16	max	.002	3	.02	2	.039	4	2.183e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	0	9	-4.395e-4	4	2319.836	2	NC	1
185		17	max	.002	3	.022	2	.041	4	2.389e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	0	9	-4.318e-4	4	2123.734	2	NC	1
187		18	max	.002	3	.024	2	.043	4	2.594e-4	3	NC	3	NC	1
188			min	-.003	2	-.02	3	0	9	-4.24e-4	4	1954.994	2	NC	1
189		19	max	.003	3	.025	2	.045	4	2.8e-4	3	NC	3	NC	1
190			min	-.004	2	-.021	3	0	9	-4.162e-4	4	1809.625	2	NC	1
191	M8	1	max	.004	2	.024	2	0	9	2.04e-3	4	NC	1	NC	1
192			min	-.001	3	-.019	3	-.047	4	-2.131e-4	3	NC	1	409.881	4
193		2	max	.004	2	.023	2	0	9	2.04e-3	4	NC	1	NC	1
194			min	-.001	3	-.018	3	-.043	4	-2.131e-4	3	NC	1	446.769	4
195		3	max	.004	2	.021	2	0	9	2.04e-3	4	NC	1	NC	1
196			min	-.001	3	-.017	3	-.039	4	-2.131e-4	3	NC	1	490.665	4
197		4	max	.004	2	.02	2	0	9	2.04e-3	4	NC	1	NC	1
198			min	-.001	3	-.015	3	-.036	4	-2.131e-4	3	NC	1	543.415	4
199		5	max	.003	2	.019	2	0	9	2.04e-3	4	NC	1	NC	1
200			min	-.001	3	-.014	3	-.032	4	-2.131e-4	3	NC	1	607.535	4
201		6	max	.003	2	.017	2	0	9	2.04e-3	4	NC	1	NC	1
202			min	0	3	-.013	3	-.028	4	-2.131e-4	3	NC	1	686.519	4
203		7	max	.003	2	.016	2	0	9	2.04e-3	4	NC	1	NC	1
204			min	0	3	-.012	3	-.025	4	-2.131e-4	3	NC	1	785.348	4
205		8	max	.003	2	.015	2	0	9	2.04e-3	4	NC	1	NC	1
206			min	0	3	-.011	3	-.021	4	-2.131e-4	3	NC	1	911.31	4
207		9	max	.002	2	.013	2	0	9	2.04e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.018	4	-2.131e-4	3	NC	1	1075.433	4
209		10	max	.002	2	.012	2	0	9	2.04e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.015	4	-2.131e-4	3	NC	1	1295.047	4
211		11	max	.002	2	.011	2	0	9	2.04e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.012	4	-2.131e-4	3	NC	1	1598.693	4
213		12	max	.002	2	.009	2	0	9	2.04e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.664e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.046e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.007	4	3.626e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.43e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.009	4	1.588e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.814e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.012	4	2.975e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-6.199e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.014	4	3.416e-6	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-6.583e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.016	5	3.858e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-6.967e-4	4	NC	1	NC	1
283		9	max	0	3	0	2	.019	5	4.299e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.003	3	-7.351e-4	4	NC	1	NC	1
285		10	max	0	3	.001	2	.021	5	4.741e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-7.735e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.023	5	5.182e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	3	-8.119e-4	4	NC	1	NC	1
289		12	max	0	3	.002	2	.025	5	5.623e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-8.503e-4	4	NC	1	NC	1
291		13	max	0	3	.003	2	.027	5	6.065e-6	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	3	-8.887e-4	4	NC	1	NC	1
293		14	max	0	3	.004	2	.029	5	6.506e-6	10	NC	1	NC	1
294			min	0	2	-.007	3	-.003	3	-9.271e-4	4	NC	1	NC	1
295		15	max	0	3	.004	2	.031	5	6.947e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.003	3	-9.655e-4	4	NC	1	NC	1
297		16	max	0	3	.005	2	.034	5	7.389e-6	10	NC	1	NC	1
298			min	0	2	-.007	3	-.003	3	-1.004e-3	4	8841.912	2	NC	1
299		17	max	0	3	.006	2	.036	5	7.83e-6	10	NC	1	NC	1
300			min	-.001	2	-.007	3	-.002	3	-1.042e-3	4	7534.578	2	NC	1
301		18	max	0	3	.007	2	.038	5	8.271e-6	10	NC	3	NC	1
302			min	-.001	2	-.007	3	-.002	3	-1.081e-3	4	6526.731	2	NC	1
303		19	max	.001	3	.008	2	.04	5	8.713e-6	10	NC	3	NC	1
304			min	-.001	2	-.007	3	-.002	3	-1.119e-3	4	5741.32	2	NC	1
305	M12	1	max	.001	1	.008	2	.002	1	2.634e-3	4	NC	1	NC	1
306			min	0	3	-.006	3	-.043	5	-9.065e-6	10	NC	1	444.896	5
307		2	max	.001	1	.007	2	.001	1	2.634e-3	4	NC	1	NC	1
308			min	0	3	-.006	3	-.04	5	-9.065e-6	10	NC	1	484.922	5
309		3	max	.001	1	.007	2	.001	1	2.634e-3	4	NC	1	NC	1
310			min	0	3	-.006	3	-.036	5	-9.065e-6	10	NC	1	532.552	5
311		4	max	.001	1	.006	2	.001	1	2.634e-3	4	NC	1	NC	1
312			min	0	3	-.005	3	-.033	5	-9.065e-6	10	NC	1	589.787	5
313		5	max	.001	1	.006	2	.001	1	2.634e-3	4	NC	1	NC	1
314			min	0	3	-.005	3	-.029	5	-9.065e-6	10	NC	1	659.355	5
315		6	max	.001	1	.005	2	0	1	2.634e-3	4	NC	1	NC	1
316			min	0	3	-.005	3	-.026	5	-9.065e-6	10	NC	1	745.05	5
317		7	max	0	1	.005	2	0	1	2.634e-3	4	NC	1	NC	1
318			min	0	3	-.004	3	-.023	5	-9.065e-6	10	NC	1	852.272	5
319		8	max	0	1	.005	2	0	1	2.634e-3	4	NC	1	NC	1
320			min	0	3	-.004	3	-.02	5	-9.065e-6	10	NC	1	988.929	5
321		9	max	0	1	.004	2	0	1	2.634e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.017	5	-9.065e-6	10	NC	1	1166.982	5
323		10	max	0	1	.004	2	0	1	2.634e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.014	5	-9.065e-6	10	NC	1	1405.229	5
325		11	max	0	1	.003	2	0	1	2.634e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.011	5	-9.065e-6	10	NC	1	1734.631	5
327		12	max	0	1	.003	2	0	1	2.634e-3	4	NC	1	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.009	5	-9.065e-6	10	NC	1	2209.187	5
329		13	max	0	1	.003	2	0	1	2.634e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.007	5	-9.065e-6	10	NC	1	2930.175	5
331		14	max	0	1	.002	2	0	1	2.634e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.005	5	-9.065e-6	10	NC	1	4106.487	5
333		15	max	0	1	.002	2	0	1	2.634e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.003	5	-9.065e-6	10	NC	1	6228.841	5
335		16	max	0	1	.001	2	0	1	2.634e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.002	5	-9.065e-6	10	NC	1	NC	1
337		17	max	0	1	0	2	0	1	2.634e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-9.065e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.634e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-9.065e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.634e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-9.065e-6	10	NC	1	NC	1
343	M1	1	max	.006	3	.022	3	.005	5	5.361e-3	2	NC	1	NC	1
344			min	-.007	2	-.018	2	0	9	-7.61e-3	3	NC	1	NC	1
345		2	max	.006	3	.012	3	.006	5	2.65e-3	2	NC	4	NC	1
346			min	-.007	2	-.01	2	-.001	9	-3.735e-3	3	5022.059	3	NC	1
347		3	max	.006	3	.003	3	.008	5	1.969e-4	5	NC	4	NC	1
348			min	-.007	2	-.002	2	-.002	1	-7.743e-5	1	2606.176	3	NC	1
349		4	max	.006	3	.005	2	.011	5	1.912e-4	5	NC	4	NC	1
350			min	-.007	2	-.004	3	-.002	1	-6.09e-5	1	1863.889	3	7935.575	5
351		5	max	.006	3	.011	2	.013	5	1.855e-4	5	NC	4	NC	1
352			min	-.007	2	-.01	3	-.002	1	-4.792e-5	9	1511.284	3	5635.708	5
353		6	max	.006	3	.016	2	.016	5	1.798e-4	5	NC	4	NC	1
354			min	-.007	2	-.015	3	-.002	1	-3.503e-5	9	1315.748	3	4304.541	5
355		7	max	.006	3	.02	2	.018	5	1.741e-4	5	NC	4	NC	1
356			min	-.007	2	-.019	3	-.002	1	-2.214e-5	9	1175.781	2	3449.195	5
357		8	max	.006	3	.023	2	.021	5	1.684e-4	5	NC	4	NC	1
358			min	-.007	2	-.022	3	-.001	1	-9.247e-6	9	1087.271	2	2860.56	5
359		9	max	.006	3	.025	2	.024	5	1.654e-4	4	NC	4	NC	1
360			min	-.007	2	-.023	3	0	9	-1.222e-6	10	1035.226	2	2435.2	4
361		10	max	.006	3	.025	2	.027	4	1.638e-4	4	NC	4	NC	1
362			min	-.007	2	-.023	3	0	9	-2.198e-6	10	1011.718	2	2102.846	4
363		11	max	.006	3	.025	2	.03	4	1.622e-4	4	NC	4	NC	1
364			min	-.007	2	-.022	3	0	10	-3.173e-6	10	1013.929	2	1849.989	4
365		12	max	.006	3	.023	2	.033	4	1.606e-4	4	NC	4	NC	1
366			min	-.007	2	-.021	3	0	10	-4.148e-6	10	1043.089	2	1653.406	4
367		13	max	.006	3	.021	2	.036	4	1.59e-4	4	NC	4	NC	1
368			min	-.007	2	-.018	3	0	10	-5.123e-6	10	1105.126	2	1498.032	4
369		14	max	.006	3	.017	2	.039	4	1.574e-4	4	NC	4	NC	1
370			min	-.007	2	-.014	3	0	10	-6.098e-6	10	1213.633	2	1373.733	4
371		15	max	.006	3	.011	2	.041	4	1.558e-4	4	NC	4	NC	1
372			min	-.007	2	-.009	3	0	10	-7.074e-6	10	1398.34	2	1273.492	4
373		16	max	.006	3	.004	2	.044	4	3.032e-4	4	NC	4	NC	1
374			min	-.007	2	-.004	3	0	10	-7.807e-6	10	1731.566	2	1192.337	4
375		17	max	.006	3	.002	3	.046	4	3.997e-3	4	NC	4	NC	1
376			min	-.007	2	-.004	2	0	10	-2.793e-6	10	2441.285	2	1126.752	4
377		18	max	.006	3	.009	3	.048	4	3.723e-3	2	NC	4	NC	1
378			min	-.007	2	-.013	2	0	10	-1.927e-3	3	4722.155	2	1073.93	4
379		19	max	.006	3	.017	3	.05	4	7.505e-3	2	NC	1	NC	1
380			min	-.007	2	-.023	2	0	9	-3.943e-3	3	NC	1	1033.083	4
381	M5	1	max	.017	3	.067	3	.005	5	1.267e-5	4	NC	1	NC	1
382			min	-.021	2	-.057	2	0	9	0	1	NC	1	NC	1
383		2	max	.017	3	.037	3	.006	5	9.742e-5	3	NC	4	NC	1
384			min	-.021	2	-.031	2	0	9	-1.03e-5	9	1610.298	3	NC	1





Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442		min	-0.007	2	-0.021	3	-0.004	3	-1.29e-4	4	1043.353	2	2223.747	5
443	13	max	.006	3	.021	2	.029	5	5.222e-6	10	NC	4	NC	1
444		min	-0.007	2	-0.018	3	-0.004	3	-1.41e-4	4	1105.412	2	1891.479	5
445	14	max	.006	3	.017	2	.033	5	6.189e-6	10	NC	4	NC	1
446		min	-0.007	2	-0.014	3	-0.003	3	-1.53e-4	4	1213.953	2	1643.886	5
447	15	max	.006	3	.011	2	.036	5	7.156e-6	10	NC	4	NC	1
448		min	-0.007	2	-0.009	3	-0.003	3	-1.65e-4	4	1398.711	2	1454.989	5
449	16	max	.006	3	.004	2	.04	5	1.493e-5	5	NC	4	NC	1
450		min	-0.007	2	-0.004	3	-0.002	3	-1.409e-4	1	1732.021	2	1308.295	5
451	17	max	.006	3	.002	3	.043	5	3.942e-3	4	NC	4	NC	1
452		min	-0.007	2	-0.004	2	-0.002	1	-6.589e-5	1	2441.878	2	1192.786	5
453	18	max	.006	3	.009	3	.047	5	1.957e-3	3	NC	4	NC	1
454		min	-0.007	2	-0.013	2	0	1	-3.723e-3	2	4723.263	2	1096.982	4
455	19	max	.006	3	.017	3	.05	4	3.941e-3	3	NC	1	NC	1
456		min	-0.007	2	-0.023	2	0	9	-7.506e-3	2	NC	1	1019.901	4
457	M13	1	max	0	.021	3	.006	3	3.621e-3	3	NC	1	NC	1
458		min	-0.004	5	-0.018	2	-0.007	2	-3.168e-3	2	NC	1	NC	1
459	2	max	0	9	.058	3	.004	3	4.48e-3	3	NC	4	NC	1
460		min	-0.004	5	-0.044	2	-0.006	2	-3.926e-3	2	2441.349	3	NC	1
461	3	max	0	9	.089	3	.005	9	5.339e-3	3	NC	4	NC	1
462		min	-0.004	5	-0.066	2	-0.006	2	-4.683e-3	2	1323.853	3	NC	1
463	4	max	0	9	.111	3	.007	9	6.197e-3	3	NC	5	NC	1
464		min	-0.004	5	-0.082	2	-0.006	2	-5.441e-3	2	1006.354	3	NC	1
465	5	max	0	9	.121	3	.007	9	7.056e-3	3	NC	5	NC	1
466		min	-0.004	5	-0.09	2	-0.008	2	-6.198e-3	2	906.571	3	NC	1
467	6	max	0	9	.119	3	.008	3	7.915e-3	3	NC	5	NC	1
468		min	-0.004	5	-0.09	2	-0.01	2	-6.956e-3	2	922.436	3	NC	1
469	7	max	0	9	.108	3	.01	3	8.774e-3	3	NC	5	NC	1
470		min	-0.004	5	-0.083	2	-0.013	2	-7.713e-3	2	1042.24	3	NC	1
471	8	max	0	9	.091	3	.013	3	9.632e-3	3	NC	4	NC	1
472		min	-0.004	5	-0.073	2	-0.017	2	-8.471e-3	2	1291.965	3	8784.906	2
473	9	max	0	9	.075	3	.015	3	1.049e-2	3	NC	4	NC	1
474		min	-0.004	5	-0.062	2	-0.02	2	-9.228e-3	2	1681.764	3	6928.675	2
475	10	max	0	9	.067	3	.017	3	1.135e-2	3	NC	4	NC	4
476		min	-0.005	5	-0.057	2	-0.021	2	-9.986e-3	2	1959.762	3	6350.521	2
477	11	max	0	9	.075	3	.019	3	1.049e-2	3	NC	4	NC	1
478		min	-0.005	5	-0.062	2	-0.02	2	-9.228e-3	2	1681.763	3	6928.697	2
479	12	max	0	9	.091	3	.019	3	9.636e-3	3	NC	4	NC	1
480		min	-0.005	5	-0.073	2	-0.017	2	-8.471e-3	2	1291.964	3	6926.608	3
481	13	max	0	9	.108	3	.018	3	8.778e-3	3	NC	5	NC	1
482		min	-0.005	5	-0.083	2	-0.013	2	-7.713e-3	2	1042.24	3	7304.621	3
483	14	max	0	9	.119	3	.017	3	7.921e-3	3	NC	5	NC	1
484		min	-0.005	5	-0.09	2	-0.01	2	-6.956e-3	2	922.436	3	8289.056	3
485	15	max	0	9	.121	3	.015	3	7.064e-3	3	NC	5	NC	1
486		min	-0.005	5	-0.09	2	-0.008	2	-6.198e-3	2	906.571	3	NC	1
487	16	max	0	9	.111	3	.012	3	6.207e-3	3	NC	5	NC	1
488		min	-0.005	5	-0.082	2	-0.006	2	-5.441e-3	2	1006.354	3	NC	1
489	17	max	0	9	.09	3	.01	3	5.35e-3	3	NC	4	NC	1
490		min	-0.005	5	-0.066	2	-0.006	2	-4.683e-3	2	1323.853	3	NC	1
491	18	max	0	9	.058	3	.008	3	4.493e-3	3	NC	4	NC	1
492		min	-0.005	5	-0.044	2	-0.006	2	-3.926e-3	2	2441.348	3	NC	1
493	19	max	0	9	.022	3	.006	3	3.636e-3	3	NC	1	NC	1
494		min	-0.005	5	-0.018	2	-0.007	2	-3.169e-3	2	NC	1	NC	1
495	M16	1	max	0	.017	3	.006	3	3.87e-3	2	NC	1	NC	1
496		min	-0.05	4	-0.023	2	-0.007	2	-2.782e-3	3	NC	1	NC	1
497	2	max	0	9	.036	3	.008	3	4.798e-3	2	NC	4	NC	1
498		min	-0.05	4	-0.06	2	-0.006	2	-3.414e-3	3	2474.967	2	NC	1





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Software
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Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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