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1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	150 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 35.33 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

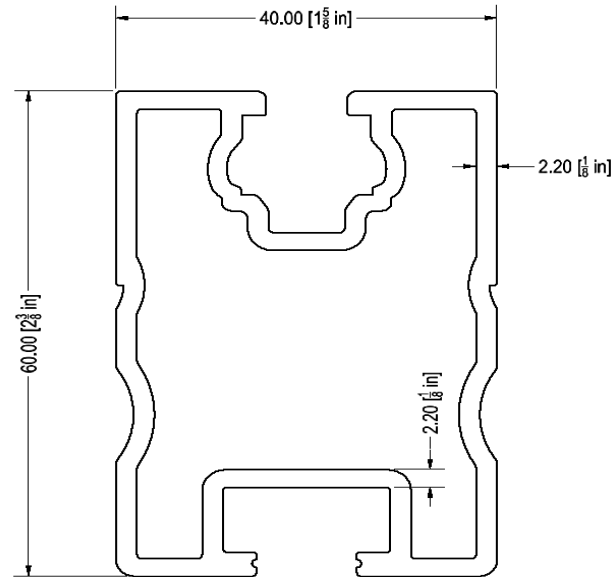
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

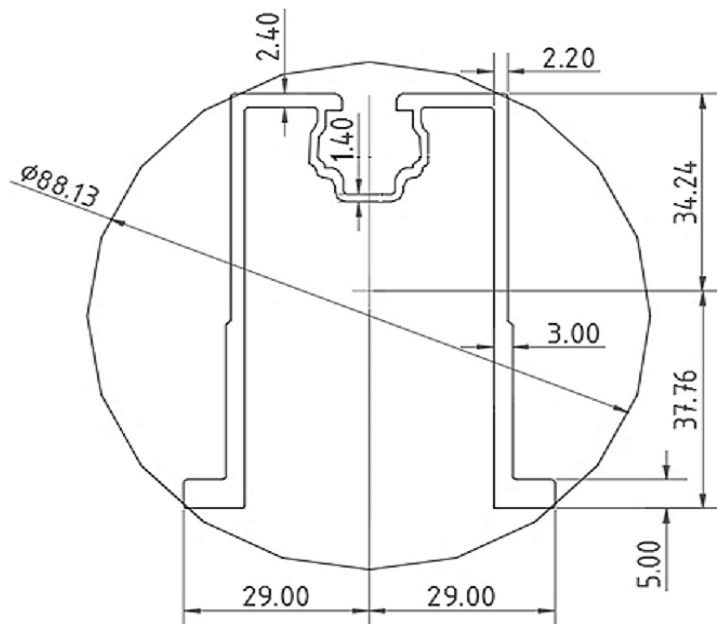
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	51 in
ΦF_{ty} STRONG-AXIS =	29.63 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.420 k-ft
M_z =	0.035 k-ft
$M_{y \text{ allowable}}$ =	1.261 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	37%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.77 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.477 k-ft
M_z =	-0.019 k-ft
P_n =	0.166 k
$M_{y \text{ allowable}}$ =	1.461 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	38%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.880 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	7%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.468 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.712 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.092 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	5%



A cross brace kit is required every 38 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

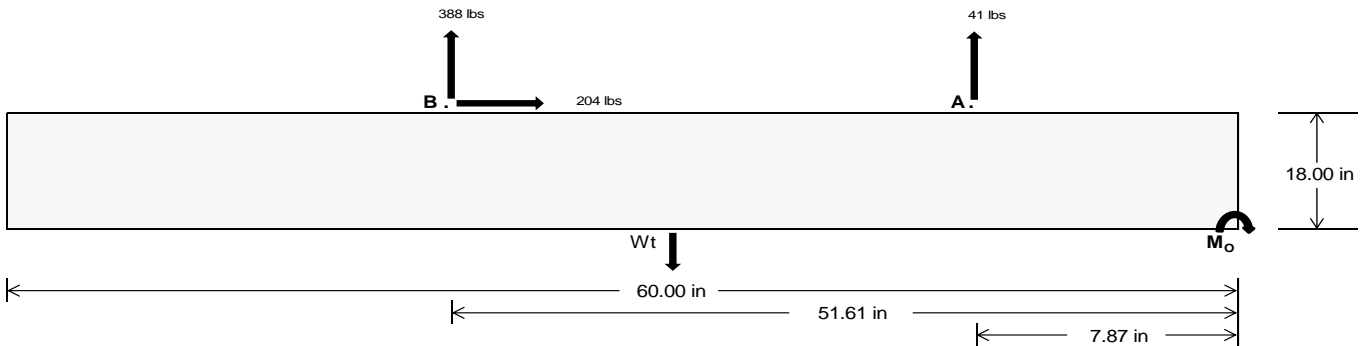
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	183.62	1686.23	k
Compressive Load =	1144.30	1124.58	k
Lateral Load =	1.68	885.47	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24044.7$ in-lbs
Resisting Force Required = 801.49 lbs
S.F. = 1.67
Weight Required = 1335.81 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 204.28 lbs
Friction = 0.4
Weight Required = 510.69 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 204.28 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	375 lbs	375 lbs	375 lbs	375 lbs	429 lbs	429 lbs	429 lbs	429 lbs	572 lbs	572 lbs	572 lbs	572 lbs	-82 lbs	-82 lbs	-82 lbs	-82 lbs
F_B	263 lbs	263 lbs	263 lbs	263 lbs	466 lbs	466 lbs	466 lbs	466 lbs	524 lbs	524 lbs	524 lbs	524 lbs	-777 lbs	-777 lbs	-777 lbs	-777 lbs
F_V	31 lbs	31 lbs	31 lbs	31 lbs	364 lbs	364 lbs	364 lbs	364 lbs	294 lbs	294 lbs	294 lbs	294 lbs	-409 lbs	-409 lbs	-409 lbs	-409 lbs
P_{total}	2541 lbs	2631 lbs	2722 lbs	2813 lbs	2799 lbs	2890 lbs	2980 lbs	3071 lbs	2999 lbs	3090 lbs	3181 lbs	3271 lbs	283 lbs	337 lbs	392 lbs	446 lbs
M	265 lbs-ft	265 lbs-ft	265 lbs-ft	265 lbs-ft	497 lbs-ft	497 lbs-ft	497 lbs-ft	497 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	634 lbs-ft	634 lbs-ft	634 lbs-ft	634 lbs-ft
e	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.24 ft	1.88 ft	1.62 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	254.0 psf	252.4 psf	250.9 psf	249.5 psf	251.7 psf	250.2 psf	248.7 psf	247.4 psf	267.2 psf	264.9 psf	262.9 psf	261.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	326.7 psf	321.7 psf	317.2 psf	313.0 psf	388.0 psf	380.3 psf	373.2 psf	366.7 psf	418.4 psf	409.2 psf	400.9 psf	393.3 psf	418.4 psf	198.1 psf	154.8 psf	138.0 psf

Maximum Bearing Pressure = 418 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

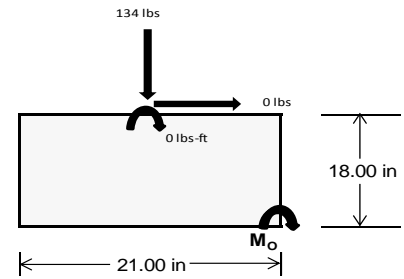
Overturning Check

$M_o = 0.0 \text{ ft-lbs}$
 Resisting Force Required = 0.00 lbs
 S.F. = 1.67
 Weight Required = 0.00 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	54 lbs	134 lbs	51 lbs	183 lbs	525 lbs	180 lbs	16 lbs	39 lbs	15 lbs
F_v	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2410 lbs	2490 lbs	2407 lbs	2426 lbs	2767 lbs	2423 lbs	705 lbs	728 lbs	704 lbs
M	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	275.4 sqft	284.5 sqft	275.1 sqft	276.9 sqft	316.1 sqft	276.7 sqft	80.5 sqft	83.2 sqft	80.4 sqft
f_{max}	275.5 psf	284.6 psf	275.1 psf	277.6 psf	316.5 psf	277.1 psf	80.6 psf	83.2 psf	80.5 psf



Maximum Bearing Pressure = 316 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

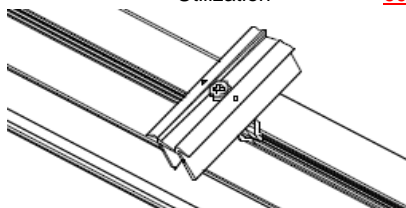
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

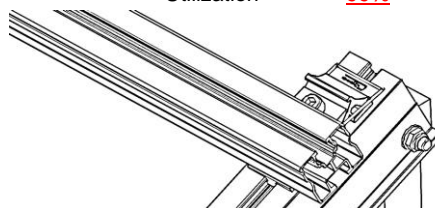
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.732 k
Allowable Uplift =	1.214 k
Utilization =	<u>60%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.109 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

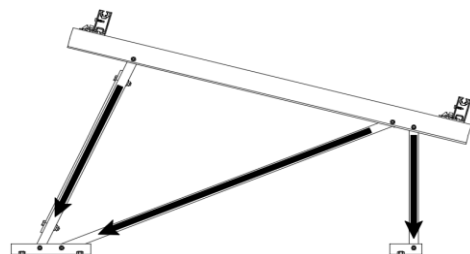
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.880 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>15%</u>

Diagonal Strut

Maximum Axial Load =	0.468 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.135 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.092 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.006 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$132.801$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$137.906$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.261 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.33 \\
 &21.3453 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.33 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.461 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	187.411	2	271.142	2	.002	10	0	10	0	1	0	1
2		min	-225.361	3	-406.535	3	-.156	3	0	3	0	1	0	1
3	N7	max	0	15	309.778	1	0	10	0	10	0	1	0	1
4		min	-.128	2	-32.422	3	-.539	1	0	1	0	1	0	1
5	N15	max	0	15	880.228	1	.167	9	0	9	0	1	0	1
6		min	-1.295	2	-141.243	3	-.588	3	0	3	0	1	0	1
7	N16	max	617.646	2	865.059	2	0	2	0	9	0	1	0	1
8		min	-681.127	3	-1297.098	3	-73.782	3	0	3	0	1	0	1
9	N23	max	0	15	309.893	1	.867	1	.001	1	0	1	0	1
10		min	-.128	2	-31.894	3	0	10	0	10	0	1	0	1
11	N24	max	187.411	2	273.618	2	74.388	3	0	1	0	1	0	1
12		min	-225.789	3	-405.506	3	-.002	10	0	3	0	1	0	1
13	Totals:	max	990.916	2	2796.644	2	0	10						
14		min	-1132.476	3	-2314.699	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	225.137	1	.644	4	.153	1	0	10	0	15	0	1
2			min	-366.377	3	.152	15	-.087	3	0	1	0	1	0	1
3		2	max	225.254	1	.599	4	.153	1	0	10	0	15	0	15
4			min	-366.29	3	.141	15	-.087	3	0	1	0	3	0	4
5		3	max	225.37	1	.553	4	.153	1	0	10	0	9	0	15
6			min	-366.203	3	.13	15	-.087	3	0	1	0	3	0	4
7		4	max	225.487	1	.507	4	.153	1	0	10	0	9	0	15
8			min	-366.115	3	.12	15	-.087	3	0	1	0	3	0	4
9		5	max	225.603	1	.462	4	.153	1	0	10	0	1	0	15
10			min	-366.028	3	.109	15	-.087	3	0	1	0	3	0	4
11		6	max	225.719	1	.416	4	.153	1	0	10	0	1	0	15
12			min	-365.941	3	.098	15	-.087	3	0	1	0	3	0	4
13		7	max	225.836	1	.37	4	.153	1	0	10	0	1	0	15
14			min	-365.854	3	.087	15	-.087	3	0	1	0	3	0	4
15		8	max	225.952	1	.325	4	.153	1	0	10	0	1	0	15
16			min	-365.766	3	.077	15	-.087	3	0	1	0	3	0	4
17		9	max	226.069	1	.279	4	.153	1	0	10	0	1	0	15
18			min	-365.679	3	.066	15	-.087	3	0	1	0	3	0	4
19		10	max	226.185	1	.233	4	.153	1	0	10	0	1	0	15
20			min	-365.592	3	.055	15	-.087	3	0	1	0	3	0	4
21		11	max	226.301	1	.188	4	.153	1	0	10	0	1	0	15
22			min	-365.504	3	.044	15	-.087	3	0	1	0	3	0	4
23		12	max	226.418	1	.142	4	.153	1	0	10	0	1	0	15
24			min	-365.417	3	.034	15	-.087	3	0	1	0	3	0	4
25		13	max	226.534	1	.105	2	.153	1	0	10	0	1	0	15
26			min	-365.33	3	.018	12	-.087	3	0	1	0	3	0	4
27		14	max	226.651	1	.069	2	.153	1	0	10	0	1	0	15
28			min	-365.242	3	-.003	3	-.087	3	0	1	0	3	0	4
29		15	max	226.767	1	.034	2	.153	1	0	10	0	1	0	15
30			min	-365.155	3	-.03	3	-.087	3	0	1	0	3	0	4
31		16	max	226.883	1	-.002	2	.153	1	0	10	0	1	0	15
32			min	-365.068	3	-.057	3	-.087	3	0	1	0	3	0	4
33		17	max	227	1	-.02	15	.153	1	0	10	0	1	0	15
34			min	-364.981	3	-.086	4	-.087	3	0	1	0	3	0	4
35		18	max	227.116	1	-.031	15	.153	1	0	10	0	1	0	15
36			min	-364.893	3	-.132	4	-.087	3	0	1	0	3	0	4
37		19	max	227.233	1	-.041	15	.153	1	0	10	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-364.806	3	-.178	4	-.087	3	0	1	0	3	0	4
39	M3	1	max	139.194	2	1.779	4	-.002	10	0	10	0	1	4
40		min	-131.821	3	.418	15	-.175	1	0	1	0	10	0	15
41		2	max	139.126	2	1.602	4	-.002	10	0	10	0	1	2
42		min	-131.872	3	.377	15	-.175	1	0	1	0	10	0	12
43		3	max	139.057	2	1.425	4	-.002	10	0	10	0	1	2
44		min	-131.924	3	.335	15	-.175	1	0	1	0	10	0	3
45		4	max	138.989	2	1.247	4	-.002	10	0	10	0	1	15
46		min	-131.975	3	.293	15	-.175	1	0	1	0	10	0	4
47		5	max	138.92	2	1.07	4	-.002	10	0	10	0	1	15
48		min	-132.026	3	.252	15	-.175	1	0	1	0	10	0	4
49		6	max	138.851	2	.893	4	-.002	10	0	10	0	1	15
50		min	-132.078	3	.21	15	-.175	1	0	1	0	10	0	4
51		7	max	138.783	2	.716	4	-.002	10	0	10	0	1	15
52		min	-132.129	3	.169	15	-.175	1	0	1	0	10	0	4
53		8	max	138.714	2	.539	4	-.002	10	0	10	0	1	15
54		min	-132.181	3	.127	15	-.175	1	0	1	0	10	-.001	4
55		9	max	138.646	2	.361	4	-.002	10	0	10	0	1	15
56		min	-132.232	3	.085	15	-.175	1	0	1	0	10	-.001	4
57		10	max	138.577	2	.184	4	-.002	10	0	10	0	1	15
58		min	-132.284	3	.044	15	-.175	1	0	1	0	10	-.001	4
59		11	max	138.508	2	.029	2	-.002	10	0	10	0	1	15
60		min	-132.335	3	-.022	3	-.175	1	0	1	0	10	-.001	4
61		12	max	138.44	2	-.04	15	-.002	10	0	10	0	1	15
62		min	-132.387	3	-.17	4	-.175	1	0	1	0	10	-.001	4
63		13	max	138.371	2	-.081	15	-.002	10	0	10	0	1	15
64		min	-132.438	3	-.347	4	-.175	1	0	1	0	10	-.001	4
65		14	max	138.303	2	-.123	15	-.002	10	0	10	0	1	15
66		min	-132.49	3	-.525	4	-.175	1	0	1	0	10	-.001	4
67		15	max	138.234	2	-.165	15	-.002	10	0	10	0	9	15
68		min	-132.541	3	-.702	4	-.175	1	0	1	0	10	0	4
69		16	max	138.165	2	-.206	15	-.002	10	0	10	0	10	15
70		min	-132.592	3	-.879	4	-.175	1	0	1	0	1	0	4
71		17	max	138.097	2	-.248	15	-.002	10	0	10	0	10	15
72		min	-132.644	3	-1.056	4	-.175	1	0	1	0	1	0	4
73		18	max	138.028	2	-.29	15	-.002	10	0	10	0	10	15
74		min	-132.695	3	-1.233	4	-.175	1	0	1	0	1	0	4
75		19	max	137.96	2	-.331	15	-.002	10	0	10	0	10	1
76		min	-132.747	3	-1.411	4	-.175	1	0	1	0	1	0	1
77	M4	1	max	308.614	1	0	1	0	10	0	1	0	3	1
78		min	-33.296	3	0	1	-.568	1	0	1	0	2	0	1
79		2	max	308.678	1	0	1	0	10	0	1	0	15	1
80		min	-33.247	3	0	1	-.568	1	0	1	0	1	0	1
81		3	max	308.743	1	0	1	0	10	0	1	0	15	1
82		min	-33.199	3	0	1	-.568	1	0	1	0	1	0	1
83		4	max	308.808	1	0	1	0	10	0	1	0	15	1
84		min	-33.15	3	0	1	-.568	1	0	1	0	1	0	1
85		5	max	308.873	1	0	1	0	10	0	1	0	15	1
86		min	-33.102	3	0	1	-.568	1	0	1	0	1	0	1
87		6	max	308.937	1	0	1	0	10	0	1	0	10	1
88		min	-33.053	3	0	1	-.568	1	0	1	0	1	0	1
89		7	max	309.002	1	0	1	0	10	0	1	0	10	1
90		min	-33.005	3	0	1	-.568	1	0	1	0	1	0	1
91		8	max	309.067	1	0	1	0	10	0	1	0	10	1
92		min	-32.956	3	0	1	-.568	1	0	1	0	1	0	1
93		9	max	309.131	1	0	1	0	10	0	1	0	10	1
94		min	-32.908	3	0	1	-.568	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	309.196	1	0	1	0	10	0	1	0	10	0	1
96		min	-32.859	3	0	1	-.568	1	0	1	0	1	0	1
97	11	max	309.261	1	0	1	0	10	0	1	0	10	0	1
98		min	-32.811	3	0	1	-.568	1	0	1	0	1	0	1
99	12	max	309.326	1	0	1	0	10	0	1	0	10	0	1
100		min	-32.762	3	0	1	-.568	1	0	1	0	1	0	1
101	13	max	309.39	1	0	1	0	10	0	1	0	10	0	1
102		min	-32.714	3	0	1	-.568	1	0	1	0	1	0	1
103	14	max	309.455	1	0	1	0	10	0	1	0	10	0	1
104		min	-32.665	3	0	1	-.568	1	0	1	0	1	0	1
105	15	max	309.52	1	0	1	0	10	0	1	0	10	0	1
106		min	-32.617	3	0	1	-.568	1	0	1	0	1	0	1
107	16	max	309.584	1	0	1	0	10	0	1	0	10	0	1
108		min	-32.568	3	0	1	-.568	1	0	1	0	1	0	1
109	17	max	309.649	1	0	1	0	10	0	1	0	10	0	1
110		min	-32.52	3	0	1	-.568	1	0	1	0	1	0	1
111	18	max	309.714	1	0	1	0	10	0	1	0	10	0	1
112		min	-32.471	3	0	1	-.568	1	0	1	0	1	0	1
113	19	max	309.778	1	0	1	0	10	0	1	0	10	0	1
114		min	-32.422	3	0	1	-.568	1	0	1	0	1	0	1
115	M6	1	max 709.746	1	.643	4	.036	9	0	3	0	3	0	1
116		min	-1135.021	3	.151	15	-.261	3	0	2	0	2	0	1
117	2	max	709.863	1	.597	4	.036	9	0	3	0	3	0	15
118		min	-1134.934	3	.141	15	-.261	3	0	2	0	2	0	4
119	3	max	709.979	1	.551	4	.036	9	0	3	0	3	0	15
120		min	-1134.846	3	.13	15	-.261	3	0	2	0	2	0	4
121	4	max	710.095	1	.506	4	.036	9	0	3	0	3	0	15
122		min	-1134.759	3	.119	15	-.261	3	0	2	0	2	0	4
123	5	max	710.212	1	.46	4	.036	9	0	3	0	3	0	15
124		min	-1134.672	3	.108	15	-.261	3	0	2	0	2	0	4
125	6	max	710.328	1	.417	2	.036	9	0	3	0	9	0	15
126		min	-1134.585	3	.098	15	-.261	3	0	2	0	3	0	4
127	7	max	710.445	1	.382	2	.036	9	0	3	0	9	0	15
128		min	-1134.497	3	.085	12	-.261	3	0	2	0	3	0	4
129	8	max	710.561	1	.346	2	.036	9	0	3	0	9	0	15
130		min	-1134.41	3	.067	12	-.261	3	0	2	0	3	0	4
131	9	max	710.677	1	.311	2	.036	9	0	3	0	9	0	15
132		min	-1134.323	3	.05	12	-.261	3	0	2	0	3	0	4
133	10	max	710.794	1	.275	2	.036	9	0	3	0	9	0	15
134		min	-1134.235	3	.032	12	-.261	3	0	2	0	3	0	4
135	11	max	710.91	1	.239	2	.036	9	0	3	0	9	0	15
136		min	-1134.148	3	.011	3	-.261	3	0	2	0	3	0	2
137	12	max	711.027	1	.204	2	.036	9	0	3	0	9	0	15
138		min	-1134.061	3	-.016	3	-.261	3	0	2	0	3	0	2
139	13	max	711.143	1	.168	2	.036	9	0	3	0	9	0	12
140		min	-1133.973	3	-.042	3	-.261	3	0	2	0	3	0	2
141	14	max	711.259	1	.133	2	.036	9	0	3	0	9	0	12
142		min	-1133.886	3	-.069	3	-.261	3	0	2	0	3	0	2
143	15	max	711.376	1	.097	2	.036	9	0	3	0	9	0	12
144		min	-1133.799	3	-.096	3	-.261	3	0	2	0	3	0	2
145	16	max	711.492	1	.062	2	.036	9	0	3	0	9	0	12
146		min	-1133.712	3	-.122	3	-.261	3	0	2	0	3	0	2
147	17	max	711.609	1	.026	2	.036	9	0	3	0	9	0	12
148		min	-1133.624	3	-.149	3	-.261	3	0	2	0	3	0	2
149	18	max	711.725	1	-.01	2	.036	9	0	3	0	9	0	12
150		min	-1133.537	3	-.176	3	-.261	3	0	2	0	3	0	2
151	19	max	711.841	1	-.042	15	.036	9	0	3	0	9	0	12



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152			min	-1133.45	3	-.202	3	-.261	3	0	2	0	3	0	2
153	M7	1	max	467.999	2	1.78	4	.021	3	0	9	0	1	0	2
154			min	-371.993	3	.419	15	-.013	1	0	3	0	3	0	12
155		2	max	467.93	2	1.603	4	.021	3	0	9	0	1	0	2
156			min	-372.044	3	.377	15	-.013	1	0	3	0	3	0	3
157		3	max	467.862	2	1.426	4	.021	3	0	9	0	1	0	2
158			min	-372.095	3	.335	15	-.013	1	0	3	0	3	0	3
159		4	max	467.793	2	1.248	4	.021	3	0	9	0	1	0	2
160			min	-372.147	3	.294	15	-.013	1	0	3	0	3	0	3
161		5	max	467.724	2	1.071	4	.021	3	0	9	0	1	0	15
162			min	-372.198	3	.252	15	-.013	1	0	3	0	3	0	3
163		6	max	467.656	2	.894	4	.021	3	0	9	0	1	0	15
164			min	-372.25	3	.21	15	-.013	1	0	3	0	3	0	4
165		7	max	467.587	2	.717	4	.021	3	0	9	0	1	0	15
166			min	-372.301	3	.169	15	-.013	1	0	3	0	3	0	4
167		8	max	467.519	2	.54	4	.021	3	0	9	0	1	0	15
168			min	-372.353	3	.127	15	-.013	1	0	3	0	3	-.001	4
169		9	max	467.45	2	.362	4	.021	3	0	9	0	1	0	15
170			min	-372.404	3	.085	15	-.013	1	0	3	0	3	-.001	4
171		10	max	467.381	2	.217	2	.021	3	0	9	0	1	0	15
172			min	-372.456	3	.025	12	-.013	1	0	3	0	3	-.001	4
173		11	max	467.313	2	.079	2	.021	3	0	9	0	1	0	15
174			min	-372.507	3	-.073	3	-.013	1	0	3	0	3	-.001	4
175		12	max	467.244	2	-.04	15	.021	3	0	9	0	1	0	15
176			min	-372.559	3	-.177	3	-.013	1	0	3	0	3	-.001	4
177		13	max	467.176	2	-.081	15	.021	3	0	9	0	1	0	15
178			min	-372.61	3	-.346	4	-.013	1	0	3	0	3	-.001	4
179		14	max	467.107	2	-.123	15	.021	3	0	9	0	1	0	15
180			min	-372.661	3	-.524	4	-.013	1	0	3	0	3	-.001	4
181		15	max	467.038	2	-.164	15	.021	3	0	9	0	1	0	15
182			min	-372.713	3	-.701	4	-.013	1	0	3	0	3	0	4
183		16	max	466.97	2	-.206	15	.021	3	0	9	0	1	0	15
184			min	-372.764	3	-.878	4	-.013	1	0	3	0	3	0	4
185		17	max	466.901	2	-.248	15	.021	3	0	9	0	1	0	15
186			min	-372.816	3	-1.055	4	-.013	1	0	3	0	3	0	4
187		18	max	466.832	2	-.289	15	.021	3	0	9	0	1	0	15
188			min	-372.867	3	-1.232	4	-.013	1	0	3	0	3	0	4
189		19	max	466.764	2	-.331	15	.021	3	0	9	0	9	0	1
190			min	-372.919	3	-1.41	4	-.013	1	0	3	0	3	0	1
191	M8	1	max	879.063	1	0	1	.178	9	0	1	0	2	0	1
192			min	-142.117	3	0	1	-.575	3	0	1	0	3	0	1
193		2	max	879.128	1	0	1	.178	9	0	1	0	9	0	1
194			min	-142.068	3	0	1	-.575	3	0	1	0	3	0	1
195		3	max	879.192	1	0	1	.178	9	0	1	0	9	0	1
196			min	-142.02	3	0	1	-.575	3	0	1	0	3	0	1
197		4	max	879.257	1	0	1	.178	9	0	1	0	9	0	1
198			min	-141.971	3	0	1	-.575	3	0	1	0	3	0	1
199		5	max	879.322	1	0	1	.178	9	0	1	0	9	0	1
200			min	-141.923	3	0	1	-.575	3	0	1	0	3	0	1
201		6	max	879.386	1	0	1	.178	9	0	1	0	9	0	1
202			min	-141.874	3	0	1	-.575	3	0	1	0	3	0	1
203		7	max	879.451	1	0	1	.178	9	0	1	0	9	0	1
204			min	-141.826	3	0	1	-.575	3	0	1	0	3	0	1
205		8	max	879.516	1	0	1	.178	9	0	1	0	9	0	1
206			min	-141.777	3	0	1	-.575	3	0	1	0	3	0	1
207		9	max	879.58	1	0	1	.178	9	0	1	0	9	0	1
208			min	-141.729	3	0	1	-.575	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	879.645	1	0	1	.178	9	0	1	0	9	0	1
210		min	-141.68	3	0	1	-.575	3	0	1	0	3	0	1
211	11	max	879.71	1	0	1	.178	9	0	1	0	9	0	1
212		min	-141.632	3	0	1	-.575	3	0	1	0	3	0	1
213	12	max	879.775	1	0	1	.178	9	0	1	0	9	0	1
214		min	-141.583	3	0	1	-.575	3	0	1	0	3	0	1
215	13	max	879.839	1	0	1	.178	9	0	1	0	9	0	1
216		min	-141.534	3	0	1	-.575	3	0	1	0	3	0	1
217	14	max	879.904	1	0	1	.178	9	0	1	0	9	0	1
218		min	-141.486	3	0	1	-.575	3	0	1	0	3	0	1
219	15	max	879.969	1	0	1	.178	9	0	1	0	9	0	1
220		min	-141.437	3	0	1	-.575	3	0	1	0	3	0	1
221	16	max	880.033	1	0	1	.178	9	0	1	0	9	0	1
222		min	-141.389	3	0	1	-.575	3	0	1	0	3	0	1
223	17	max	880.098	1	0	1	.178	9	0	1	0	9	0	1
224		min	-141.34	3	0	1	-.575	3	0	1	0	3	0	1
225	18	max	880.163	1	0	1	.178	9	0	1	0	9	0	1
226		min	-141.292	3	0	1	-.575	3	0	1	0	3	0	1
227	19	max	880.228	1	0	1	.178	9	0	1	0	9	0	1
228		min	-141.243	3	0	1	-.575	3	0	1	0	3	0	1
229	M10	1	max	226.777	1	.644	4	-.003	15	0	1	0	1	0
230		min	-312.981	3	.152	15	-.109	1	0	3	0	3	0	1
231	2	max	226.893	1	.599	4	-.003	15	0	1	0	1	0	15
232		min	-312.894	3	.141	15	-.109	1	0	3	0	3	0	4
233	3	max	227.01	1	.553	4	-.003	15	0	1	0	1	0	15
234		min	-312.807	3	.13	15	-.109	1	0	3	0	3	0	4
235	4	max	227.126	1	.507	4	-.003	15	0	1	0	1	0	15
236		min	-312.719	3	.12	15	-.109	1	0	3	0	3	0	4
237	5	max	227.243	1	.462	4	-.003	15	0	1	0	1	0	15
238		min	-312.632	3	.109	15	-.109	1	0	3	0	3	0	4
239	6	max	227.359	1	.416	4	-.003	15	0	1	0	1	0	15
240		min	-312.545	3	.098	15	-.109	1	0	3	0	3	0	4
241	7	max	227.475	1	.37	4	-.003	15	0	1	0	9	0	15
242		min	-312.457	3	.087	15	-.109	1	0	3	0	3	0	4
243	8	max	227.592	1	.325	4	-.003	15	0	1	0	10	0	15
244		min	-312.37	3	.077	15	-.109	1	0	3	0	3	0	4
245	9	max	227.708	1	.279	4	-.003	15	0	1	0	10	0	15
246		min	-312.283	3	.066	15	-.109	1	0	3	0	3	0	4
247	10	max	227.825	1	.233	4	-.003	15	0	1	0	10	0	15
248		min	-312.195	3	.055	15	-.109	1	0	3	0	3	0	4
249	11	max	227.941	1	.188	4	-.003	15	0	1	0	10	0	15
250		min	-312.108	3	.044	15	-.109	1	0	3	0	3	0	4
251	12	max	228.057	1	.142	4	-.003	15	0	1	0	10	0	15
252		min	-312.021	3	.034	15	-.109	1	0	3	0	3	0	4
253	13	max	228.174	1	.105	2	-.003	15	0	1	0	10	0	15
254		min	-311.934	3	.023	15	-.109	1	0	3	0	3	0	4
255	14	max	228.29	1	.069	2	-.003	15	0	1	0	10	0	15
256		min	-311.846	3	.012	15	-.109	1	0	3	0	3	0	4
257	15	max	228.407	1	.034	2	-.003	15	0	1	0	10	0	15
258		min	-311.759	3	-.007	3	-.109	1	0	3	0	3	0	4
259	16	max	228.523	1	-.002	2	-.003	15	0	1	0	10	0	15
260		min	-311.672	3	-.041	4	-.109	1	0	3	0	3	0	4
261	17	max	228.639	1	-.02	15	-.003	15	0	1	0	10	0	15
262		min	-311.584	3	-.086	4	-.109	1	0	3	0	3	0	4
263	18	max	228.756	1	-.031	15	-.003	15	0	1	0	10	0	15
264		min	-311.497	3	-.132	4	-.109	1	0	3	0	3	0	4
265	19	max	228.872	1	-.041	15	-.003	15	0	1	0	10	0	15



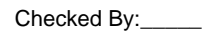
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-311.41	3	-.178	4	-.109	1	0	3	0	3	0	4
267	M11	1	max	138.766	2	1.779	4	.187	1	0	3	0	3	0	4
268			min	-132.544	3	.418	15	-.04	3	0	10	0	1	0	15
269		2	max	138.697	2	1.602	4	.187	1	0	3	0	3	0	2
270			min	-132.595	3	.377	15	-.04	3	0	10	0	1	0	12
271		3	max	138.628	2	1.425	4	.187	1	0	3	0	3	0	2
272			min	-132.647	3	.335	15	-.04	3	0	10	0	1	0	3
273		4	max	138.56	2	1.247	4	.187	1	0	3	0	3	0	15
274			min	-132.698	3	.293	15	-.04	3	0	10	0	1	0	3
275		5	max	138.491	2	1.07	4	.187	1	0	3	0	3	0	15
276			min	-132.75	3	.252	15	-.04	3	0	10	0	1	0	4
277		6	max	138.423	2	.893	4	.187	1	0	3	0	3	0	15
278			min	-132.801	3	.21	15	-.04	3	0	10	0	1	0	4
279		7	max	138.354	2	.716	4	.187	1	0	3	0	3	0	15
280			min	-132.852	3	.169	15	-.04	3	0	10	0	1	0	4
281		8	max	138.285	2	.539	4	.187	1	0	3	0	3	0	15
282			min	-132.904	3	.127	15	-.04	3	0	10	0	1	-.001	4
283		9	max	138.217	2	.361	4	.187	1	0	3	0	3	0	15
284			min	-132.955	3	.085	15	-.04	3	0	10	0	1	-.001	4
285		10	max	138.148	2	.184	4	.187	1	0	3	0	3	0	15
286			min	-133.007	3	.044	15	-.04	3	0	10	0	1	-.001	4
287		11	max	138.08	2	.029	2	.187	1	0	3	0	3	0	15
288			min	-133.058	3	-.034	3	-.04	3	0	10	0	1	-.001	4
289		12	max	138.011	2	-.04	15	.187	1	0	3	0	3	0	15
290			min	-133.11	3	-.17	4	-.04	3	0	10	0	1	-.001	4
291		13	max	137.942	2	-.081	15	.187	1	0	3	0	3	0	15
292			min	-133.161	3	-.347	4	-.04	3	0	10	0	1	-.001	4
293		14	max	137.874	2	-.123	15	.187	1	0	3	0	3	0	15
294			min	-133.213	3	-.525	4	-.04	3	0	10	0	1	-.001	4
295		15	max	137.805	2	-.165	15	.187	1	0	3	0	3	0	15
296			min	-133.264	3	-.702	4	-.04	3	0	10	0	2	0	4
297		16	max	137.737	2	-.206	15	.187	1	0	3	0	3	0	15
298			min	-133.316	3	-.879	4	-.04	3	0	10	0	10	0	4
299		17	max	137.668	2	-.248	15	.187	1	0	3	0	3	0	15
300			min	-133.367	3	-1.056	4	-.04	3	0	10	0	10	0	4
301		18	max	137.599	2	-.29	15	.187	1	0	3	0	3	0	15
302			min	-133.418	3	-1.233	4	-.04	3	0	10	0	10	0	4
303		19	max	137.531	2	-.331	15	.187	1	0	3	0	3	0	1
304			min	-133.47	3	-1.411	4	-.04	3	0	10	0	10	0	1
305	M12	1	max	308.728	1	0	1	.913	1	0	1	0	2	0	1
306			min	-32.767	3	0	1	0	10	0	1	0	3	0	1
307		2	max	308.793	1	0	1	.913	1	0	1	0	1	0	1
308			min	-32.719	3	0	1	0	10	0	1	0	15	0	1
309		3	max	308.858	1	0	1	.913	1	0	1	0	1	0	1
310			min	-32.67	3	0	1	0	10	0	1	0	15	0	1
311		4	max	308.923	1	0	1	.913	1	0	1	0	1	0	1
312			min	-32.622	3	0	1	0	10	0	1	0	10	0	1
313		5	max	308.987	1	0	1	.913	1	0	1	0	1	0	1
314			min	-32.573	3	0	1	0	10	0	1	0	10	0	1
315		6	max	309.052	1	0	1	.913	1	0	1	0	1	0	1
316			min	-32.524	3	0	1	0	10	0	1	0	10	0	1
317		7	max	309.117	1	0	1	.913	1	0	1	0	1	0	1
318			min	-32.476	3	0	1	0	10	0	1	0	10	0	1
319		8	max	309.181	1	0	1	.913	1	0	1	0	1	0	1
320			min	-32.427	3	0	1	0	10	0	1	0	10	0	1
321		9	max	309.246	1	0	1	.913	1	0	1	0	1	0	1
322			min	-32.379	3	0	1	0	10	0	1	0	10	0	1





Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-76.579	1	-155.2	3	-20.805	1	0	2	-.04	1	0	3
381	M5	1	max	185.084	1	1113.127	3	0	2	0	.009	3	0	3
382		min	.045	3	-743.315	2	-66.769	3	0	3	0	10	0	2
383		2	max	185.202	1	1112.937	3	0	2	0	0	9	.161	2
384		min	.133	3	-743.568	2	-66.769	3	0	3	-.005	3	-.241	3
385		3	max	165.703	3	5.401	9	7.271	3	0	0	9	.319	2
386		min	-25.064	10	-69.435	2	-.206	9	0	1	-.019	3	-.477	3
387		4	max	165.791	3	5.191	9	7.271	3	0	0	9	.334	2
388		min	-24.965	10	-69.689	2	-.206	9	0	1	-.017	3	-.463	3
389		5	max	165.88	3	4.98	9	7.271	3	0	0	9	.35	2
390		min	-24.867	10	-69.942	2	-.206	9	0	1	-.016	3	-.449	3
391		6	max	165.968	3	4.769	9	7.271	3	0	0	9	.365	2
392		min	-24.769	10	-70.195	2	-.206	9	0	1	-.014	3	-.435	3
393		7	max	166.057	3	4.558	9	7.271	3	0	0	9	.38	2
394		min	-24.67	10	-70.448	2	-.206	9	0	1	-.013	3	-.421	3
395		8	max	166.145	3	4.347	9	7.271	3	0	0	9	.395	2
396		min	-24.572	10	-70.701	2	-.206	9	0	1	-.011	3	-.406	3
397		9	max	166.234	3	4.136	9	7.271	3	0	0	1	.411	2
398		min	-24.474	10	-70.954	2	-.206	9	0	1	-.009	3	-.392	3
399		10	max	166.322	3	3.925	9	7.271	3	0	0	2	.426	2
400		min	-24.375	10	-71.207	2	-.206	9	0	1	-.008	3	-.378	3
401		11	max	166.411	3	3.714	9	7.271	3	0	0	2	.442	2
402		min	-24.277	10	-71.46	2	-.206	9	0	1	-.006	3	-.364	3
403		12	max	166.499	3	3.503	9	7.271	3	0	0	2	.457	2
404		min	-24.179	10	-71.713	2	-.206	9	0	1	-.005	3	-.349	3
405		13	max	166.588	3	3.292	9	7.271	3	0	0	2	.473	2
406		min	-24.08	10	-71.966	2	-.206	9	0	1	-.003	3	-.335	3
407		14	max	166.676	3	3.082	9	7.271	3	0	0	2	.488	2
408		min	-23.982	10	-72.219	2	-.206	9	0	1	-.001	3	-.32	3
409		15	max	166.765	3	2.871	9	7.271	3	0	0	3	.504	2
410		min	-23.884	10	-72.472	2	-.206	9	0	1	0	9	-.306	3
411		16	max	277.785	2	294.666	2	7.238	3	0	.001	3	.517	2
412		min	-65.141	3	-358.678	3	-.205	9	0	2	0	9	-.289	3
413		17	max	277.903	2	294.413	2	7.238	3	0	.003	3	.453	2
414		min	-65.053	3	-358.868	3	-.205	9	0	2	0	9	-.211	3
415		18	max	-3.679	12	1051.979	2	6.667	3	0	.004	3	.227	2
416		min	-185.237	1	-488.406	3	-.045	1	0	9	0	9	-.106	3
417		19	max	-3.62	12	1051.726	2	6.667	3	0	.006	3	0	3
418		min	-185.119	1	-488.595	3	-.045	1	0	9	0	9	0	2
419	M9	1	max	76.457	1	345.41	3	70.848	3	0	0	10	0	2
420		min	2.836	15	-232.724	2	.189	10	0	2	-.039	1	0	3
421		2	max	76.575	1	345.22	3	70.848	3	0	0	10	.051	2
422		min	2.872	15	-232.977	2	.189	10	0	2	-.035	1	-.075	3
423		3	max	60.35	3	4.455	9	19.769	1	0	.014	3	.1	2
424		min	-8.499	10	-19.869	2	-2.539	3	0	10	-.03	1	-.148	3
425		4	max	60.438	3	4.244	9	19.769	1	0	.013	3	.105	2
426		min	-8.401	10	-20.122	2	-2.539	3	0	10	-.026	1	-.145	3
427		5	max	60.527	3	4.033	9	19.769	1	0	.013	3	.109	2
428		min	-8.302	10	-20.375	2	-2.539	3	0	10	-.022	1	-.141	3
429		6	max	60.615	3	3.822	9	19.769	1	0	.012	3	.114	2
430		min	-8.204	10	-20.628	2	-2.539	3	0	10	-.017	1	-.137	3
431		7	max	60.704	3	3.611	9	19.769	1	0	.012	3	.118	2
432		min	-8.106	10	-20.881	2	-2.539	3	0	10	-.013	1	-.133	3
433		8	max	60.792	3	3.4	9	19.769	1	0	.011	3	.123	2
434		min	-8.007	10	-21.134	2	-2.539	3	0	10	-.009	1	-.129	3
435		9	max	60.881	3	3.189	9	19.769	1	0	.01	3	.127	2
436		min	-7.909	10	-21.387	2	-2.539	3	0	10	-.004	1	-.125	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	60.969	3	2.978	9	19.769	1	0	1	.01	3	.132	2
438		min	-7.811	10	-21.64	2	-2.539	3	0	10	0	1	-.121	3
439	11	max	61.058	3	2.768	9	19.769	1	0	1	.009	3	.137	2
440		min	-7.712	10	-21.893	2	-2.539	3	0	10	0	10	-.117	3
441	12	max	61.146	3	2.557	9	19.769	1	0	1	.009	3	.141	2
442		min	-7.614	10	-22.146	2	-2.539	3	0	10	0	10	-.112	3
443	13	max	61.235	3	2.346	9	19.769	1	0	1	.013	1	.146	2
444		min	-7.516	10	-22.399	2	-2.539	3	0	10	0	10	-.108	3
445	14	max	61.323	3	2.135	9	19.769	1	0	1	.017	1	.151	2
446		min	-7.417	10	-22.652	2	-2.539	3	0	10	0	10	-.104	3
447	15	max	61.412	3	1.924	9	19.769	1	0	1	.021	1	.156	2
448		min	-7.319	10	-22.905	2	-2.539	3	0	10	0	10	-.1	3
449	16	max	86.835	2	83.989	2	19.937	1	0	10	.026	1	.16	2
450		min	-21.074	3	-124.886	3	-2.57	3	0	3	0	10	-.094	3
451	17	max	86.953	2	83.736	2	19.937	1	0	10	.03	1	.142	2
452		min	-20.986	3	-125.076	3	-2.57	3	0	3	0	10	-.067	3
453	18	max	-2.871	15	328.871	2	20.852	1	0	2	.035	1	.072	2
454		min	-76.555	1	-155.002	3	-2.138	3	0	3	0	10	-.034	3
455	19	max	-2.835	15	328.618	2	20.852	1	0	2	.039	1	0	2
456		min	-76.437	1	-155.192	3	-2.138	3	0	3	0	10	0	3
457	M13	1	max	70.843	3	232.638	2	-2.836	15	0	.039	1	0	2
458		min	.189	10	-345.449	3	-76.452	1	0	3	0	10	0	3
459	2	max	70.843	3	165.54	2	-2.154	15	0	2	.012	3	.139	3
460		min	.189	10	-245.229	3	-57.667	1	0	3	-.002	10	-.094	2
461	3	max	70.843	3	98.442	2	-1.472	15	0	2	.009	3	.232	3
462		min	.189	10	-145.01	3	-38.881	1	0	3	-.015	1	-.156	2
463	4	max	70.843	3	31.345	2	-.186	10	0	2	.006	3	.276	3
464		min	.189	10	-44.791	3	-20.096	1	0	3	-.029	1	-.187	2
465	5	max	70.843	3	55.429	3	2.144	2	0	2	.004	3	.274	3
466		min	.189	10	-35.753	2	-4.593	3	0	3	-.034	1	-.186	2
467	6	max	70.843	3	155.648	3	17.475	1	0	2	.002	3	.224	3
468		min	.189	10	-102.851	2	-3.6	3	0	3	-.03	1	-.153	2
469	7	max	70.843	3	255.867	3	36.26	1	0	2	0	3	.127	3
470		min	.189	10	-169.949	2	-2.607	3	0	3	-.018	1	-.089	2
471	8	max	70.843	3	356.087	3	55.045	1	0	2	.005	2	.009	1
472		min	.189	10	-237.047	2	-1.614	3	0	3	0	3	-.018	3
473	9	max	70.843	3	456.306	3	73.831	1	0	2	.034	1	.135	2
474		min	.189	10	-304.144	2	-.621	3	0	3	-.001	3	-.209	3
475	10	max	70.843	3	556.526	3	92.616	1	0	2	.074	1	.295	2
476		min	.189	10	-371.242	2	.372	3	0	3	-.011	3	-.449	3
477	11	max	20.187	1	304.144	2	1.341	3	0	3	.034	1	.135	2
478		min	.189	10	-456.306	3	-73.693	1	0	2	-.01	3	-.209	3
479	12	max	20.187	1	237.047	2	2.334	3	0	3	.005	2	.009	1
480		min	.189	10	-356.087	3	-54.908	1	0	2	-.009	3	-.018	3
481	13	max	20.187	1	169.949	2	3.327	3	0	3	0	10	.127	3
482		min	.189	10	-255.867	3	-36.123	1	0	2	-.018	1	-.089	2
483	14	max	20.187	1	102.851	2	4.32	3	0	3	-.001	15	.224	3
484		min	.189	10	-155.648	3	-17.337	1	0	2	-.03	1	-.153	2
485	15	max	20.187	1	35.753	2	5.313	3	0	3	-.001	15	.274	3
486		min	.189	10	-55.429	3	-2.144	2	0	2	-.034	1	-.186	2
487	16	max	20.187	1	44.791	3	20.233	1	0	3	0	12	.276	3
488		min	.189	10	-31.345	2	.186	10	0	2	-.029	1	-.187	2
489	17	max	20.187	1	145.01	3	39.019	1	0	3	.002	3	.232	3
490		min	.189	10	-98.442	2	1.48	15	0	2	-.015	1	-.156	2
491	18	max	20.187	1	245.229	3	57.804	1	0	3	.008	1	.139	3
492		min	.189	10	-165.54	2	2.163	15	0	2	-.002	10	-.094	2
493	19	max	20.187	1	345.449	3	76.589	1	0	3	.04	1	0	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	.189	10	-232.638	2	2.845	15	0	2	0	10	0	3
495		max	2.14	3	328.723	2	-2.835	15	0	3	.039	1	0	2
496		min	-20.815	1	-155.214	3	-76.442	1	0	2	0	10	0	3
497		2 max	2.14	3	233.712	2	-2.153	15	0	3	.008	1	.063	3
498		min	-20.815	1	-110.821	3	-57.657	1	0	2	-.002	10	-.133	2
499		3 max	2.14	3	138.701	2	-1.471	15	0	3	0	3	.105	3
500		min	-20.815	1	-66.428	3	-38.871	1	0	2	-.015	1	-.221	2
501		4 max	2.14	3	43.69	2	-.191	10	0	3	-.001	15	.126	3
502		min	-20.815	1	-22.035	3	-20.086	1	0	2	-.029	1	-.264	2
503		5 max	2.14	3	22.357	3	2.134	2	0	3	-.001	15	.125	3
504		min	-20.815	1	-51.321	2	-3.002	3	0	2	-.034	1	-.262	2
505		6 max	2.14	3	66.75	3	17.485	1	0	3	-.001	15	.104	3
506		min	-20.815	1	-146.331	2	-2.009	3	0	2	-.03	1	-.215	2
507		7 max	2.14	3	111.143	3	36.27	1	0	3	0	10	.062	3
508		min	-20.815	1	-241.342	2	-1.016	3	0	2	-.018	1	-.124	2
509		8 max	2.14	3	155.536	3	55.055	1	0	3	.005	2	.013	2
510		min	-20.815	1	-336.353	2	-.023	3	0	2	-.006	3	0	3
511		9 max	2.14	3	199.928	3	73.841	1	0	3	.034	1	.194	2
512		min	-20.815	1	-431.364	2	.836	12	0	2	-.006	3	-.084	3
513		10 max	-.187	10	-8.113	15	92.626	1	0	15	.074	1	.42	2
514		min	-20.815	1	-526.375	2	-3.009	3	0	2	-.005	3	-.189	3
515		11 max	-.187	10	431.364	2	-1.46	12	0	2	.034	1	.194	2
516		min	-20.769	1	-199.928	3	-73.698	1	0	3	0	3	-.084	3
517		12 max	-.187	10	336.353	2	-.799	12	0	2	.005	2	.013	2
518		min	-20.769	1	-155.536	3	-54.913	1	0	3	-.001	3	0	3
519		13 max	-.187	10	241.342	2	-.03	3	0	2	0	10	.062	3
520		min	-20.769	1	-111.143	3	-36.128	1	0	3	-.018	1	-.124	2
521		14 max	-.187	10	146.331	2	.962	3	0	2	0	12	.104	3
522		min	-20.769	1	-66.75	3	-17.342	1	0	3	-.03	1	-.215	2
523		15 max	-.187	10	51.321	2	2.058	9	0	2	0	12	.125	3
524		min	-20.769	1	-22.357	3	-2.134	2	0	3	-.034	1	-.262	2
525		16 max	-.187	10	22.035	3	20.228	1	0	2	0	3	.126	3
526		min	-20.769	1	-43.69	2	.191	10	0	3	-.029	1	-.264	2
527		17 max	-.187	10	66.428	3	39.014	1	0	2	.002	3	.105	3
528		min	-20.769	1	-138.701	2	1.479	15	0	3	-.015	1	-.221	2
529		18 max	-.187	10	110.821	3	57.799	1	0	2	.008	1	.063	3
530		min	-20.769	1	-233.712	2	2.162	15	0	3	-.002	10	-.133	2
531		19 max	-.187	10	155.214	3	76.584	1	0	2	.04	1	0	2
532		min	-20.769	1	-328.723	2	2.844	15	0	3	0	10	0	3
533	M15	1 max	0	1	.879	3	.124	3	0	1	0	1	0	1
534		min	-90.83	3	0	1	0	1	0	3	0	3	0	1
535		2 max	0	1	.781	3	.124	3	0	1	0	1	0	1
536		min	-90.895	3	0	1	0	1	0	3	0	3	0	3
537		3 max	0	1	.684	3	.124	3	0	1	0	1	0	1
538		min	-90.961	3	0	1	0	1	0	3	0	3	0	3
539		4 max	0	1	.586	3	.124	3	0	1	0	1	0	1
540		min	-91.026	3	0	1	0	1	0	3	0	3	0	3
541		5 max	0	1	.488	3	.124	3	0	1	0	1	0	1
542		min	-91.091	3	0	1	0	1	0	3	0	3	0	3
543		6 max	0	1	.391	3	.124	3	0	1	0	1	0	1
544		min	-91.156	3	0	1	0	1	0	3	0	3	0	3
545		7 max	0	1	.293	3	.124	3	0	1	0	3	0	1
546		min	-91.221	3	0	1	0	1	0	3	0	1	-.001	3
547		8 max	0	1	.195	3	.124	3	0	1	0	3	0	1
548		min	-91.287	3	0	1	0	1	0	3	0	1	-.001	3
549		9 max	0	1	.098	3	.124	3	0	1	0	3	0	1
550		min	-91.352	3	0	1	0	1	0	3	0	1	-.001	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.124	3	0	1	0	3	0	1
552		min	-91.417	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.124	3	0	1	0	3	0	1
554		min	-91.482	3	-.098	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.124	3	0	1	0	3	0	1
556		min	-91.547	3	-.195	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.124	3	0	1	0	3	0	1
558		min	-91.613	3	-.293	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.124	3	0	1	0	3	0	1
560		min	-91.678	3	-.391	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.124	3	0	1	0	3	0	1
562		min	-91.743	3	-.488	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.124	3	0	1	0	3	0	1
564		min	-91.808	3	-.586	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.124	3	0	1	0	3	0	1
566		min	-91.873	3	-.684	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.124	3	0	1	0	3	0	1
568		min	-91.938	3	-.781	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.124	3	0	1	0	3	0	1
570		min	-92.004	3	-.879	3	0	1	0	3	0	1	0	1
571	M16A	1	max	2	1.504	4	.039	1	0	3	0	3	0	1
572		min	-90.658	3	0	2	-.05	3	0	1	0	1	0	1
573	2	max	0	2	1.337	4	.039	1	0	3	0	3	0	2
574		min	-90.593	3	0	2	-.05	3	0	1	0	1	0	4
575	3	max	0	2	1.17	4	.039	1	0	3	0	3	0	2
576		min	-90.528	3	0	2	-.05	3	0	1	0	1	0	4
577	4	max	0	2	1.003	4	.039	1	0	3	0	3	0	2
578		min	-90.463	3	0	2	-.05	3	0	1	0	1	-.001	4
579	5	max	0	2	.836	4	.039	1	0	3	0	3	0	2
580		min	-90.398	3	0	2	-.05	3	0	1	0	1	-.001	4
581	6	max	0	2	.669	4	.039	1	0	3	0	3	0	2
582		min	-90.332	3	0	2	-.05	3	0	1	0	1	-.002	4
583	7	max	0	2	.501	4	.039	1	0	3	0	3	0	2
584		min	-90.267	3	0	2	-.05	3	0	1	0	1	-.002	4
585	8	max	0	2	.334	4	.039	1	0	3	0	3	0	2
586		min	-90.202	3	0	2	-.05	3	0	1	0	1	-.002	4
587	9	max	0	2	.167	4	.039	1	0	3	0	3	0	2
588		min	-90.137	3	0	2	-.05	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.039	1	0	3	0	3	0	2
590		min	-90.072	3	0	1	-.05	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.039	1	0	3	0	3	0	2
592		min	-90.007	3	-.167	4	-.05	3	0	1	0	1	-.002	4
593	12	max	.044	13	0	2	.039	1	0	3	0	3	0	2
594		min	-89.941	3	-.334	4	-.05	3	0	1	0	1	-.002	4
595	13	max	.134	13	0	2	.039	1	0	3	0	1	0	2
596		min	-89.876	3	-.501	4	-.05	3	0	1	0	4	-.002	4
597	14	max	.224	13	0	2	.039	1	0	3	0	1	0	2
598		min	-89.811	3	-.669	4	-.05	3	0	1	0	3	-.002	4
599	15	max	.313	13	0	2	.039	1	0	3	0	1	0	2
600		min	-89.746	3	-.836	4	-.05	3	0	1	0	3	-.001	4
601	16	max	.403	13	0	2	.039	1	0	3	0	1	0	2
602		min	-89.681	3	-1.003	4	-.05	3	0	1	0	3	-.001	4
603	17	max	.493	13	0	2	.039	1	0	3	0	1	0	2
604		min	-89.615	3	-1.17	4	-.05	3	0	1	0	3	0	4
605	18	max	.589	4	0	2	.039	1	0	3	0	1	0	2
606		min	-89.55	3	-1.337	4	-.05	3	0	1	0	3	0	4
607	19	max	.701	4	0	2	.039	1	0	3	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-89.485	3	-1.504	4	-.05	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.008	2	.003	1	-2.712e-6	10	NC	3	NC	1	
2			min	-.003	3	-.008	3	-.002	3	-3.111e-4	1	4524.743	2	NC	1	
3			2	max	.002	1	.007	2	.003	1	-2.59e-6	10	NC	3	NC	1
4				min	-.003	3	-.007	3	-.002	3	-2.975e-4	1	4929.118	2	NC	1
5			3	max	.002	1	.007	2	.003	1	-2.468e-6	10	NC	3	NC	1
6				min	-.003	3	-.007	3	-.002	3	-2.839e-4	1	5408.483	2	NC	1
7			4	max	.002	1	.006	2	.003	1	-2.346e-6	10	NC	1	NC	1
8				min	-.003	3	-.007	3	-.001	3	-2.703e-4	1	5980.745	2	NC	1
9			5	max	.002	1	.005	2	.002	1	-2.224e-6	10	NC	1	NC	1
10				min	-.003	3	-.006	3	-.001	3	-2.566e-4	1	6669.722	2	NC	1
11		6	max	.001	1	.005	2	.002	1	-2.102e-6	10	NC	1	NC	1	
12			min	-.002	3	-.006	3	-.001	3	-2.43e-4	1	7507.614	2	NC	1	
13		7	max	.001	1	.004	2	.002	1	-1.98e-6	10	NC	1	NC	1	
14			min	-.002	3	-.006	3	0	3	-2.294e-4	1	8538.79	2	NC	1	
15		8	max	.001	1	.004	2	.002	1	-1.858e-6	10	NC	1	NC	1	
16			min	-.002	3	-.005	3	0	3	-2.158e-4	1	9825.769	2	NC	1	
17		9	max	.001	1	.003	2	.001	1	-1.736e-6	10	NC	1	NC	1	
18			min	-.002	3	-.005	3	0	3	-2.021e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.001	1	-1.614e-6	10	NC	1	NC	1	
20			min	-.002	3	-.005	3	0	3	-1.885e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	.001	1	-1.492e-6	10	NC	1	NC	1	
22			min	-.001	3	-.004	3	0	3	-1.749e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	0	1	-1.371e-6	10	NC	1	NC	1	
24			min	-.001	3	-.004	3	0	3	-1.613e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	-1.249e-6	10	NC	1	NC	1	
26			min	-.001	3	-.003	3	0	3	-1.477e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	-1.127e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-1.34e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-1.005e-6	10	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-1.204e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-8.828e-7	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-1.068e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-7.609e-7	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-9.316e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-6.389e-7	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-7.954e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-5.17e-7	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-6.592e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	3.073e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	2.409e-7	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	4.028e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	3.582e-7	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	4.982e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	4.754e-7	10	NC	1	NC	1
45			4	max	0	3	0	2	0	10	5.937e-5	1	NC	1	NC	1
46				min	0	2	-.002	3	0	1	5.927e-7	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	6.891e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	7.1e-7	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	7.846e-5	1	NC	1	NC	1
50				min	0	2	-.004	3	0	9	8.272e-7	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	8.8e-5	1	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	9.445e-7	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	9.755e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	1.062e-6	10	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	1.071e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	15	1.179e-6	10	NC	1	NC	1
57		10	max	0	3	.002	2	0	1	1.166e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	1.296e-6	10	NC	1	NC	1
59		11	max	0	3	.002	2	0	1	1.262e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	1.414e-6	10	NC	1	NC	1
61		12	max	0	3	.003	2	0	1	1.357e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	1.531e-6	10	NC	1	NC	1
63		13	max	.001	3	.003	2	0	1	1.453e-4	1	NC	1	NC	1
64			min	-.001	2	-.007	3	0	10	1.648e-6	10	NC	1	NC	1
65		14	max	.001	3	.004	2	.001	1	1.548e-4	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	10	1.765e-6	10	NC	1	NC	1
67		15	max	.001	3	.005	2	.001	1	1.644e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	1.883e-6	10	9674.547	2	NC	1
69		16	max	.001	3	.006	2	.002	1	1.739e-4	1	NC	1	NC	1
70			min	-.001	2	-.008	3	0	10	2.e-6	10	8156.831	2	NC	1
71		17	max	.001	3	.007	2	.002	1	1.835e-4	1	NC	1	NC	1
72			min	-.001	2	-.008	3	0	10	2.117e-6	10	6990.62	2	NC	1
73		18	max	.001	3	.008	2	.002	1	1.93e-4	1	NC	3	NC	1
74			min	-.001	2	-.008	3	0	10	2.234e-6	10	6083.569	2	NC	1
75		19	max	.002	3	.009	2	.002	1	2.026e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	2.352e-6	10	5371.337	2	NC	1
77	M4	1	max	.001	1	.009	2	0	10	-2.164e-6	10	NC	1	NC	1
78			min	0	3	-.008	3	-.002	1	-2.478e-4	1	NC	1	NC	1
79		2	max	.001	1	.009	2	0	10	-2.164e-6	10	NC	1	NC	1
80			min	0	3	-.007	3	-.002	1	-2.478e-4	1	NC	1	NC	1
81		3	max	.001	1	.008	2	0	10	-2.164e-6	10	NC	1	NC	1
82			min	0	3	-.007	3	-.002	1	-2.478e-4	1	NC	1	NC	1
83		4	max	.001	1	.008	2	0	10	-2.164e-6	10	NC	1	NC	1
84			min	0	3	-.006	3	-.001	1	-2.478e-4	1	NC	1	NC	1
85		5	max	.001	1	.007	2	0	10	-2.164e-6	10	NC	1	NC	1
86			min	0	3	-.006	3	-.001	1	-2.478e-4	1	NC	1	NC	1
87		6	max	.001	1	.007	2	0	10	-2.164e-6	10	NC	1	NC	1
88			min	0	3	-.006	3	-.001	1	-2.478e-4	1	NC	1	NC	1
89		7	max	0	1	.006	2	0	10	-2.164e-6	10	NC	1	NC	1
90			min	0	3	-.005	3	0	1	-2.478e-4	1	NC	1	NC	1
91		8	max	0	1	.006	2	0	10	-2.164e-6	10	NC	1	NC	1
92			min	0	3	-.005	3	0	1	-2.478e-4	1	NC	1	NC	1
93		9	max	0	1	.005	2	0	10	-2.164e-6	10	NC	1	NC	1
94			min	0	3	-.004	3	0	1	-2.478e-4	1	NC	1	NC	1
95		10	max	0	1	.005	2	0	10	-2.164e-6	10	NC	1	NC	1
96			min	0	3	-.004	3	0	1	-2.478e-4	1	NC	1	NC	1
97		11	max	0	1	.004	2	0	10	-2.164e-6	10	NC	1	NC	1
98			min	0	3	-.003	3	0	1	-2.478e-4	1	NC	1	NC	1
99		12	max	0	1	.004	2	0	10	-2.164e-6	10	NC	1	NC	1
100			min	0	3	-.003	3	0	1	-2.478e-4	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	10	-2.164e-6	10	NC	1	NC	1
102			min	0	3	-.003	3	0	1	-2.478e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	10	-2.164e-6	10	NC	1	NC	1
104			min	0	3	-.002	3	0	1	-2.478e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	10	-2.164e-6	10	NC	1	NC	1
106			min	0	3	-.002	3	0	1	-2.478e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	10	-2.164e-6	10	NC	1	NC	1
108			min	0	3	-.001	3	0	1	-2.478e-4	1	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	-2.164e-6	10	NC	1	NC	1
110			min	0	3	0	3	0	1	-2.478e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	-2.164e-6	10	NC	1	NC	1
112			min	0	3	0	3	0	1	-2.478e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.164e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-2.478e-4	1	NC	1	NC	1
115	M6	1	max	.006	1	.026	2	.001	9	4.076e-4	3	NC	3	NC	1
116			min	-.01	3	-.023	3	-.005	3	-8.236e-8	2	1380.738	2	6747.974	3
117		2	max	.006	1	.025	2	0	9	3.959e-4	3	NC	3	NC	1
118			min	-.01	3	-.022	3	-.005	3	-3.755e-7	11	1476.759	2	7189.562	3
119		3	max	.006	1	.023	2	0	9	3.843e-4	3	NC	3	NC	1
120			min	-.009	3	-.021	3	-.005	3	-1.262e-6	11	1586.71	2	7711.135	3
121		4	max	.005	1	.021	2	0	9	3.726e-4	3	NC	3	NC	1
122			min	-.008	3	-.02	3	-.004	3	-2.837e-6	1	1713.389	2	8330.186	3
123		5	max	.005	1	.02	2	0	9	3.61e-4	3	NC	3	NC	1
124			min	-.008	3	-.018	3	-.004	3	-4.828e-6	1	1860.405	2	9069.861	3
125		6	max	.005	1	.018	2	0	9	3.493e-4	3	NC	3	NC	1
126			min	-.007	3	-.017	3	-.004	3	-6.819e-6	1	2032.481	2	9961.257	3
127		7	max	.004	1	.016	2	0	9	3.377e-4	3	NC	3	NC	1
128			min	-.007	3	-.016	3	-.003	3	-8.811e-6	1	2235.923	2	NC	1
129		8	max	.004	1	.015	2	0	9	3.26e-4	3	NC	3	NC	1
130			min	-.006	3	-.015	3	-.003	3	-1.08e-5	1	2479.337	2	NC	1
131		9	max	.004	1	.013	2	0	9	3.144e-4	3	NC	3	NC	1
132			min	-.006	3	-.013	3	-.003	3	-1.279e-5	1	2774.779	2	NC	1
133		10	max	.003	1	.012	2	0	9	3.027e-4	3	NC	3	NC	1
134			min	-.005	3	-.012	3	-.002	3	-1.478e-5	1	3139.666	2	NC	1
135		11	max	.003	1	.01	2	0	9	2.911e-4	3	NC	3	NC	1
136			min	-.005	3	-.011	3	-.002	3	-1.678e-5	1	3600.128	2	NC	1
137		12	max	.002	1	.009	2	0	1	2.795e-4	3	NC	3	NC	1
138			min	-.004	3	-.01	3	-.002	3	-1.877e-5	1	4197.227	2	NC	1
139		13	max	.002	1	.007	2	0	1	2.678e-4	3	NC	3	NC	1
140			min	-.003	3	-.008	3	-.001	3	-2.076e-5	1	4999.401	2	NC	1
141		14	max	.002	1	.006	2	0	1	2.562e-4	3	NC	3	NC	1
142			min	-.003	3	-.007	3	-.001	3	-2.275e-5	1	6129.825	2	NC	1
143		15	max	.001	1	.005	2	0	1	2.445e-4	3	NC	1	NC	1
144			min	-.002	3	-.006	3	0	3	-2.474e-5	1	7834.837	2	NC	1
145		16	max	.001	1	.003	2	0	1	2.329e-4	3	NC	1	NC	1
146			min	-.002	3	-.004	3	0	3	-2.673e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	2.212e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-2.872e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.096e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-3.071e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.979e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-3.271e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.516e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.165e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.375e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.944e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.234e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.724e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	1.094e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-2.503e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	.002	3	9.526e-6	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-2.824e-6	3	9270.493	2	NC	1
163		6	max	.001	3	.006	2	.002	3	1.938e-5	3	NC	1	NC	1
164			min	-.001	2	-.009	3	0	1	0	2	7431.354	2	NC	1
165		7	max	.001	3	.007	2	.002	3	4.159e-5	3	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.01	3	0	1	0	5	6171.395	2	NC	1
167		8	max	.002	3	.009	2	.002	3	6.38e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-3.023e-7	4	5246.116	2	NC	1
169		9	max	.002	3	.01	2	.003	3	8.6e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	1	-1.669e-6	9	4533.765	2	NC	1
171		10	max	.002	3	.012	2	.003	3	1.082e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	1	-3.156e-6	9	3966.879	2	NC	1
173		11	max	.002	3	.013	2	.003	3	1.304e-4	3	NC	3	NC	1
174			min	-.003	2	-.016	3	0	1	-4.642e-6	9	3504.933	2	NC	1
175		12	max	.003	3	.015	2	.003	3	1.526e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	0	1	-6.129e-6	9	3121.964	2	NC	1
177		13	max	.003	3	.016	2	.003	3	1.748e-4	3	NC	3	NC	1
178			min	-.004	2	-.018	3	0	1	-7.615e-6	9	2800.42	2	NC	1
179		14	max	.003	3	.018	2	.003	3	1.97e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	0	1	-9.102e-6	9	2527.895	2	NC	1
181		15	max	.003	3	.02	2	.003	3	2.192e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	0	1	-1.059e-5	9	2295.28	2	NC	1
183		16	max	.004	3	.022	2	.003	3	2.414e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	0	1	-1.208e-5	9	2095.685	2	NC	1
185		17	max	.004	3	.024	2	.003	3	2.637e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	0	9	-1.356e-5	9	1923.765	2	NC	1
187		18	max	.004	3	.026	2	.003	3	2.859e-4	3	NC	3	NC	1
188			min	-.005	2	-.023	3	0	9	-1.505e-5	9	1775.297	2	NC	1
189		19	max	.004	3	.028	2	.003	3	3.081e-4	3	NC	3	NC	1
190			min	-.005	2	-.024	3	0	9	-1.654e-5	9	1646.898	2	NC	1
191	M8	1	max	.004	1	.03	2	0	9	-9.356e-8	10	NC	1	NC	1
192			min	0	3	-.024	3	-.002	3	-2.313e-4	3	NC	1	NC	1
193		2	max	.004	1	.028	2	0	9	-9.356e-8	10	NC	1	NC	1
194			min	0	3	-.022	3	-.002	3	-2.313e-4	3	NC	1	NC	1
195		3	max	.004	1	.026	2	0	9	-9.356e-8	10	NC	1	NC	1
196			min	0	3	-.021	3	-.002	3	-2.313e-4	3	NC	1	NC	1
197		4	max	.003	1	.025	2	0	9	-9.356e-8	10	NC	1	NC	1
198			min	0	3	-.02	3	-.001	3	-2.313e-4	3	NC	1	NC	1
199		5	max	.003	1	.023	2	0	9	-9.356e-8	10	NC	1	NC	1
200			min	0	3	-.018	3	-.001	3	-2.313e-4	3	NC	1	NC	1
201		6	max	.003	1	.021	2	0	9	-9.356e-8	10	NC	1	NC	1
202			min	0	3	-.017	3	-.001	3	-2.313e-4	3	NC	1	NC	1
203		7	max	.003	1	.02	2	0	9	-9.356e-8	10	NC	1	NC	1
204			min	0	3	-.016	3	0	3	-2.313e-4	3	NC	1	NC	1
205		8	max	.003	1	.018	2	0	9	-9.356e-8	10	NC	1	NC	1
206			min	0	3	-.014	3	0	3	-2.313e-4	3	NC	1	NC	1
207		9	max	.002	1	.017	2	0	9	-9.356e-8	10	NC	1	NC	1
208			min	0	3	-.013	3	0	3	-2.313e-4	3	NC	1	NC	1
209		10	max	.002	1	.015	2	0	9	-9.356e-8	10	NC	1	NC	1
210			min	0	3	-.012	3	0	3	-2.313e-4	3	NC	1	NC	1
211		11	max	.002	1	.013	2	0	9	-9.356e-8	10	NC	1	NC	1
212			min	0	3	-.01	3	0	3	-2.313e-4	3	NC	1	NC	1
213		12	max	.002	1	.012	2	0	9	-9.356e-8	10	NC	1	NC	1
214			min	0	3	-.009	3	0	3	-2.313e-4	3	NC	1	NC	1
215		13	max	.001	1	.01	2	0	9	-9.356e-8	10	NC	1	NC	1
216			min	0	3	-.008	3	0	3	-2.313e-4	3	NC	1	NC	1
217		14	max	.001	1	.008	2	0	9	-9.356e-8	10	NC	1	NC	1
218			min	0	3	-.007	3	0	3	-2.313e-4	3	NC	1	NC	1
219		15	max	0	1	.007	2	0	9	-9.356e-8	10	NC	1	NC	1
220			min	0	3	-.005	3	0	3	-2.313e-4	3	NC	1	NC	1
221		16	max	0	1	.005	2	0	9	-9.356e-8	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-2.313e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	9	-9.356e-8	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.313e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	9	-9.356e-8	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-2.313e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-9.356e-8	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.313e-4	3	NC	1	NC	1
229	M10	1	max	.002	1	.008	2	0	3	3.139e-4	1	NC	3	NC	1
230			min	-.003	3	-.008	3	-.001	1	-4.953e-4	3	4530.212	2	NC	1
231		2	max	.002	1	.007	2	0	3	2.985e-4	1	NC	3	NC	1
232			min	-.003	3	-.007	3	-.001	1	-4.793e-4	3	4935.224	2	NC	1
233		3	max	.002	1	.007	2	0	3	2.83e-4	1	NC	3	NC	1
234			min	-.002	3	-.007	3	-.001	1	-4.633e-4	3	5415.372	2	NC	1
235		4	max	.002	1	.006	2	0	3	2.675e-4	1	NC	1	NC	1
236			min	-.002	3	-.007	3	-.001	1	-4.473e-4	3	5988.605	2	NC	1
237		5	max	.002	1	.005	2	0	3	2.521e-4	1	NC	1	NC	1
238			min	-.002	3	-.007	3	-.001	1	-4.313e-4	3	6678.798	2	NC	1
239		6	max	.001	1	.005	2	0	3	2.366e-4	1	NC	1	NC	1
240			min	-.002	3	-.006	3	-.001	1	-4.153e-4	3	7518.231	2	NC	1
241		7	max	.001	1	.004	2	0	3	2.212e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	0	1	-3.993e-4	3	8551.39	2	NC	1
243		8	max	.001	1	.004	2	0	3	2.057e-4	1	NC	1	NC	1
244			min	-.002	3	-.006	3	0	1	-3.833e-4	3	9840.963	2	NC	1
245		9	max	.001	1	.003	2	0	3	1.903e-4	1	NC	1	NC	1
246			min	-.002	3	-.005	3	0	1	-3.673e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	1.748e-4	1	NC	1	NC	1
248			min	-.001	3	-.005	3	0	1	-3.513e-4	3	NC	1	NC	1
249		11	max	0	1	.002	2	0	3	1.594e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-3.353e-4	3	NC	1	NC	1
251		12	max	0	1	.002	2	0	3	1.439e-4	1	NC	1	NC	1
252			min	-.001	3	-.004	3	0	1	-3.193e-4	3	NC	1	NC	1
253		13	max	0	1	.001	2	0	3	1.285e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-3.033e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.13e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-2.873e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	9.756e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-2.713e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	8.21e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.553e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	6.665e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.393e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	5.12e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.233e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.574e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.073e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	9.662e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.686e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.467e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.925e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.272e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-4.164e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	11	3.077e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.403e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	2	8.819e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-6.643e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	2	-7.827e-7	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-7.882e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-8.912e-7	10	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-9.121e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	-9.998e-7	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-1.036e-4	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	-1.108e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-1.16e-4	1	NC	1	NC	1
285		10	max	0	3	.002	2	0	10	-1.217e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-1.284e-4	1	NC	1	NC	1
287		11	max	0	3	.002	2	0	10	-1.325e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-1.408e-4	1	NC	1	NC	1
289		12	max	0	3	.003	2	0	10	-1.434e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-1.532e-4	1	NC	1	NC	1
291		13	max	.001	3	.003	2	0	10	-1.542e-6	10	NC	1	NC	1
292			min	-.001	2	-.007	3	-.003	3	-1.668e-4	3	NC	1	NC	1
293		14	max	.001	3	.004	2	0	10	-1.651e-6	10	NC	1	NC	1
294			min	-.001	2	-.008	3	-.003	3	-1.887e-4	3	NC	1	NC	1
295		15	max	.001	3	.005	2	0	10	-1.76e-6	10	NC	1	NC	1
296			min	-.001	2	-.008	3	-.003	3	-2.107e-4	3	9688.504	2	NC	1
297		16	max	.001	3	.006	2	0	10	-1.868e-6	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.003	1	-2.326e-4	3	8167.362	2	NC	1
299		17	max	.001	3	.007	2	0	10	-1.977e-6	10	NC	1	NC	1
300			min	-.001	2	-.008	3	-.003	1	-2.546e-4	3	6998.799	2	NC	1
301		18	max	.001	3	.008	2	0	10	-2.085e-6	10	NC	3	NC	1
302			min	-.001	2	-.008	3	-.003	1	-2.765e-4	3	6090.098	2	NC	1
303		19	max	.002	3	.009	2	0	10	-2.194e-6	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.003	1	-2.985e-4	3	5376.688	2	NC	1
305	M12	1	max	.001	1	.009	2	.003	1	3.115e-4	3	NC	1	NC	2
306			min	0	3	-.008	3	0	10	1.977e-6	10	NC	1	6612.163	1
307		2	max	.001	1	.009	2	.003	1	3.115e-4	3	NC	1	NC	2
308			min	0	3	-.007	3	0	10	1.977e-6	10	NC	1	7211.767	1
309		3	max	.001	1	.008	2	.002	1	3.115e-4	3	NC	1	NC	2
310			min	0	3	-.007	3	0	10	1.977e-6	10	NC	1	7925.462	1
311		4	max	.001	1	.008	2	.002	1	3.115e-4	3	NC	1	NC	2
312			min	0	3	-.007	3	0	10	1.977e-6	10	NC	1	8783.329	1
313		5	max	.001	1	.007	2	.002	1	3.115e-4	3	NC	1	NC	2
314			min	0	3	-.006	3	0	10	1.977e-6	10	NC	1	9826.345	1
315		6	max	.001	1	.007	2	.002	1	3.115e-4	3	NC	1	NC	1
316			min	0	3	-.006	3	0	10	1.977e-6	10	NC	1	NC	1
317		7	max	0	1	.006	2	.002	1	3.115e-4	3	NC	1	NC	1
318			min	0	3	-.005	3	0	10	1.977e-6	10	NC	1	NC	1
319		8	max	0	1	.006	2	.001	1	3.115e-4	3	NC	1	NC	1
320			min	0	3	-.005	3	0	10	1.977e-6	10	NC	1	NC	1
321		9	max	0	1	.005	2	.001	1	3.115e-4	3	NC	1	NC	1
322			min	0	3	-.004	3	0	10	1.977e-6	10	NC	1	NC	1
323		10	max	0	1	.005	2	0	1	3.115e-4	3	NC	1	NC	1
324			min	0	3	-.004	3	0	10	1.977e-6	10	NC	1	NC	1
325		11	max	0	1	.004	2	0	1	3.115e-4	3	NC	1	NC	1
326			min	0	3	-.003	3	0	10	1.977e-6	10	NC	1	NC	1
327		12	max	0	1	.004	2	0	1	3.115e-4	3	NC	1	NC	1
328			min	0	3	-.003	3	0	10	1.977e-6	10	NC	1	NC	1
329		13	max	0	1	.003	2	0	1	3.115e-4	3	NC	1	NC	1
330			min	0	3	-.003	3	0	10	1.977e-6	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	3.115e-4	3	NC	1	NC	1
332			min	0	3	-.002	3	0	10	1.977e-6	10	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.115e-4	3	NC	1	NC	1
334			min	0	3	-.002	3	0	10	1.977e-6	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	3.115e-4	3	NC	1	NC	1
336			min	0	3	-.001	3	0	10	1.977e-6	10	NC	1	NC	1



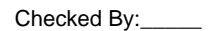
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.115e-4	3	NC	1	NC	1
338			min	0	3	0	3	0	10	1.977e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.115e-4	3	NC	1	NC	1
340			min	0	3	0	3	0	10	1.977e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.115e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	1.977e-6	10	NC	1	NC	1
343	M1	1	max	.007	3	.023	3	.003	3	6.681e-3	2	NC	1	NC	1
344			min	-.008	2	-.02	2	-.001	9	-9.542e-3	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.002	3	3.293e-3	2	NC	4	NC	1
346			min	-.008	2	-.011	2	-.002	1	-4.699e-3	3	4835.977	3	NC	1
347		3	max	.007	3	.004	3	.002	3	5.459e-5	3	NC	4	NC	1
348			min	-.008	2	-.003	2	-.003	1	-1.625e-4	1	2508.32	3	NC	1
349		4	max	.007	3	.004	2	.001	3	5.47e-5	3	NC	4	NC	1
350			min	-.008	2	-.004	3	-.004	1	-1.352e-4	1	1792.041	3	NC	1
351		5	max	.007	3	.011	2	.001	3	5.48e-5	3	NC	4	NC	1
352			min	-.008	2	-.01	3	-.004	1	-1.079e-4	1	1451.421	3	NC	1
353		6	max	.007	3	.016	2	0	3	5.49e-5	3	NC	4	NC	1
354			min	-.008	2	-.015	3	-.004	1	-8.061e-5	1	1258.068	2	NC	1
355		7	max	.007	3	.02	2	0	3	5.501e-5	3	NC	4	NC	1
356			min	-.008	2	-.019	3	-.003	1	-5.331e-5	1	1121.846	2	NC	1
357		8	max	.007	3	.023	2	0	3	5.511e-5	3	NC	4	NC	1
358			min	-.008	2	-.022	3	-.003	1	-3.007e-5	9	1036.56	2	NC	1
359		9	max	.007	3	.025	2	0	3	5.521e-5	3	NC	5	NC	1
360			min	-.008	2	-.023	3	-.002	1	-1.023e-5	9	986.194	2	NC	1
361		10	max	.007	3	.026	2	0	3	5.532e-5	3	NC	5	NC	1
362			min	-.008	2	-.023	3	0	9	7.442e-7	15	963.121	2	NC	1
363		11	max	.007	3	.026	2	0	3	5.588e-5	1	NC	4	NC	1
364			min	-.008	2	-.023	3	0	9	1.212e-6	10	964.607	2	NC	1
365		12	max	.007	3	.024	2	0	1	8.318e-5	1	NC	4	NC	1
366			min	-.008	2	-.021	3	0	10	1.401e-6	10	991.796	2	NC	1
367		13	max	.007	3	.021	2	.002	1	1.105e-4	1	NC	4	NC	1
368			min	-.008	2	-.018	3	0	10	1.591e-6	10	1050.315	2	NC	1
369		14	max	.007	3	.017	2	.002	1	1.378e-4	1	NC	4	NC	1
370			min	-.008	2	-.014	3	0	10	1.78e-6	10	1153.119	2	NC	1
371		15	max	.007	3	.011	2	.002	1	1.651e-4	1	NC	4	NC	1
372			min	-.008	2	-.009	3	0	10	1.97e-6	10	1328.608	2	NC	1
373		16	max	.007	3	.004	2	.002	1	1.849e-4	1	NC	4	NC	1
374			min	-.008	2	-.003	3	0	10	2.117e-6	10	1646.138	2	NC	1
375		17	max	.007	3	.003	3	.002	1	5.452e-5	3	NC	4	NC	1
376			min	-.008	2	-.005	2	0	10	5.576e-7	15	2326.663	2	NC	1
377		18	max	.007	3	.011	3	0	1	4.622e-3	2	NC	4	NC	1
378			min	-.008	2	-.015	2	0	10	-2.299e-3	3	4505.275	2	NC	1
379		19	max	.007	3	.018	3	0	3	9.313e-3	2	NC	1	NC	1
380			min	-.008	2	-.026	2	0	1	-4.698e-3	3	NC	1	NC	1
381	M5	1	max	.022	3	.074	3	.003	3	5.465e-6	3	NC	1	NC	1
382			min	-.025	2	-.064	2	-.001	9	0	11	NC	1	NC	1
383		2	max	.022	3	.042	3	.004	3	1.115e-4	3	NC	4	NC	1
384			min	-.025	2	-.036	2	-.001	9	-1.975e-5	9	1529.063	3	NC	1
385		3	max	.022	3	.013	3	.005	3	2.154e-4	3	NC	5	NC	1
386			min	-.025	2	-.009	2	-.001	9	-3.921e-5	9	793.46	3	NC	1
387		4	max	.022	3	.014	2	.006	3	2.098e-4	3	NC	5	NC	1
388			min	-.025	2	-.012	3	-.001	9	-3.728e-5	9	567.516	3	NC	1
389		5	max	.022	3	.034	2	.007	3	2.041e-4	3	NC	5	NC	1
390			min	-.025	2	-.032	3	0	9	-3.535e-5	9	460.161	2	NC	1
391		6	max	.022	3	.051	2	.007	3	1.984e-4	3	NC	5	NC	1
392			min	-.025	2	-.048	3	0	9	-3.342e-5	9	391.198	2	9718.469	3
393		7	max	.021	3	.064	2	.007	3	1.927e-4	3	NC	5	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451	17	max	.007	3	.003	3	0	10	3.53e-5	3	NC	4	NC	1
452		min	-.008	2	-.005	2	-.003	1	-1.104e-4	1	2327.31	2	NC	1
453	18	max	.007	3	.011	3	0	10	2.345e-3	3	NC	4	NC	1
454		min	-.008	2	-.015	2	-.002	1	-4.622e-3	2	4506.486	2	NC	1
455	19	max	.007	3	.018	3	0	3	4.696e-3	3	NC	1	NC	1
456		min	-.008	2	-.026	2	0	9	-9.313e-3	2	NC	1	NC	1
457	M13	1	max	9	.023	3	.007	3	3.76e-3	3	NC	1	NC	1
458		min	-.003	3	-.02	2	-.008	2	-3.302e-3	2	NC	1	NC	1
459	2	max	.001	9	.075	3	.005	3	4.67e-3	3	NC	4	NC	1
460		min	-.003	3	-.056	2	-.006	2	-4.112e-3	2	1949.082	3	NC	1
461	3	max	.001	9	.119	3	.009	9	5.58e-3	3	NC	5	NC	2
462		min	-.003	3	-.087	2	-.005	2	-4.922e-3	2	1061.336	3	7912.701	1
463	4	max	.001	9	.148	3	.014	1	6.49e-3	3	NC	5	NC	2
464		min	-.003	3	-.109	2	-.005	10	-5.732e-3	2	812.757	3	5670.633	1
465	5	max	.001	9	.161	3	.015	1	7.4e-3	3	NC	5	NC	2
466		min	-.003	3	-.118	2	-.007	10	-6.542e-3	2	740.836	3	5273.049	1
467	6	max	.001	9	.156	3	.014	9	8.31e-3	3	NC	5	NC	2
468		min	-.003	3	-.116	2	-.009	2	-7.352e-3	2	767.99	3	6260.694	1
469	7	max	.001	9	.137	3	.014	3	9.22e-3	3	NC	5	NC	1
470		min	-.003	3	-.105	2	-.014	2	-8.162e-3	2	894.402	3	NC	1
471	8	max	.001	9	.111	3	.016	3	1.013e-2	3	NC	4	NC	1
472		min	-.003	3	-.088	2	-.019	2	-8.972e-3	2	1165.646	3	9149.272	2
473	9	max	.001	9	.085	3	.019	3	1.104e-2	3	NC	4	NC	1
474		min	-.003	3	-.071	2	-.023	2	-9.782e-3	2	1636.5	3	6552.551	2
475	10	max	.001	9	.074	3	.022	3	1.195e-2	3	NC	4	NC	4
476		min	-.003	3	-.064	2	-.025	2	-1.059e-2	2	2014.703	3	5824.413	2
477	11	max	.001	9	.086	3	.023	3	1.104e-2	3	NC	4	NC	1
478		min	-.003	3	-.071	2	-.023	2	-9.782e-3	2	1636.498	3	6280.576	3
479	12	max	.001	9	.111	3	.024	3	1.013e-2	3	NC	4	NC	1
480		min	-.003	3	-.088	2	-.019	2	-8.972e-3	2	1165.645	3	6107.53	3
481	13	max	.001	9	.137	3	.023	3	9.225e-3	3	NC	5	NC	1
482		min	-.003	3	-.105	2	-.014	2	-8.162e-3	2	894.401	3	6398.315	3
483	14	max	.001	9	.156	3	.021	3	8.317e-3	3	NC	5	NC	2
484		min	-.003	3	-.116	2	-.009	2	-7.352e-3	2	767.99	3	6255.933	1
485	15	max	.001	9	.161	3	.019	3	7.408e-3	3	NC	5	NC	2
486		min	-.003	3	-.118	2	-.007	10	-6.542e-3	2	740.836	3	5277.448	1
487	16	max	.001	9	.149	3	.016	3	6.5e-3	3	NC	5	NC	2
488		min	-.003	3	-.109	2	-.005	10	-5.732e-3	2	812.756	3	5683.328	1
489	17	max	.001	9	.119	3	.012	3	5.592e-3	3	NC	5	NC	2
490		min	-.003	3	-.087	2	-.005	2	-4.922e-3	2	1061.335	3	7944.266	1
491	18	max	.001	9	.076	3	.009	3	4.684e-3	3	NC	4	NC	1
492		min	-.003	3	-.056	2	-.006	2	-4.112e-3	2	1949.081	3	NC	1
493	19	max	.001	9	.023	3	.007	3	3.775e-3	3	NC	1	NC	1
494		min	-.003	3	-.02	2	-.008	2	-3.303e-3	2	NC	1	NC	1
495	M16	1	max	9	.018	3	.007	3	4.063e-3	2	NC	1	NC	1
496		min	0	3	-.026	2	-.008	2	-2.917e-3	3	NC	1	NC	1
497	2	max	0	9	.044	3	.01	3	5.065e-3	2	NC	4	NC	1
498		min	0	3	-.077	2	-.006	2	-3.597e-3	3	1996.432	2	NC	1
499	3	max	0	9	.066	3	.012	3	6.066e-3	2	NC	5	NC	2
500		min	0	3	-.12	2	-.005	2	-4.276e-3	3	1084.747	2	7947.561	1
501	4	max	0	9	.082	3	.015	3	7.068e-3	2	NC	5	NC	2
502		min	0	3	-.149	2	-.005	10	-4.956e-3	3	827.48	2	5697.196	1
503	5	max	0	9	.089	3	.018	3	8.069e-3	2	NC	5	NC	2
504		min	0	3	-.162	2	-.007	10	-5.635e-3	3	749.586	2	5303.132	1
505	6	max	0	9	.089	3	.02	3	9.07e-3	2	NC	5	NC	2
506		min	0	3	-.158	2	-.009	2	-6.314e-3	3	769.39	2	6312.416	1
507	7	max	0	9	.082	3	.021	3	1.007e-2	2	NC	5	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.141	2	-.014	2	-6.994e-3	3	881.543	2	7079.823	3
509	8	max	0	9	.072	3	.022	3	1.107e-2	2	NC	4	NC	1
510		min	0	3	-.117	2	-.019	2	-7.673e-3	3	1117.715	2	6766.841	3
511	9	max	0	9	.062	3	.022	3	1.207e-2	2	NC	4	NC	1
512		min	0	3	-.093	2	-.023	2	-8.353e-3	3	1503.479	2	6549.596	2
513	10	max	0	9	.057	3	.021	3	1.308e-2	2	NC	4	NC	4
514		min	0	3	-.083	2	-.025	2	-9.032e-3	3	1792.27	2	5822.328	2
515	11	max	0	9	.062	3	.02	3	1.207e-2	2	NC	4	NC	1
516		min	0	3	-.093	2	-.023	2	-8.351e-3	3	1503.479	2	6549.608	2
517	12	max	0	9	.072	3	.019	3	1.107e-2	2	NC	4	NC	1
518		min	0	3	-.117	2	-.019	2	-7.67e-3	3	1117.715	2	8849.946	3
519	13	max	0	9	.082	3	.017	3	1.007e-2	2	NC	5	NC	1
520		min	0	3	-.141	2	-.014	2	-6.989e-3	3	881.543	2	NC	1
521	14	max	0	9	.089	3	.015	3	9.071e-3	2	NC	5	NC	2
522		min	0	3	-.158	2	-.009	2	-6.308e-3	3	769.39	2	6320.59	1
523	15	max	0	9	.089	3	.015	1	8.07e-3	2	NC	5	NC	2
524		min	0	3	-.162	2	-.007	10	-5.627e-3	3	749.586	2	5317.077	1
525	16	max	0	1	.082	3	.013	1	7.069e-3	2	NC	5	NC	2
526		min	0	3	-.149	2	-.005	10	-4.946e-3	3	827.48	2	5720.199	1
527	17	max	0	1	.066	3	.01	3	6.067e-3	2	NC	5	NC	2
528		min	0	3	-.12	2	-.005	2	-4.265e-3	3	1084.747	2	7995.33	1
529	18	max	0	1	.044	3	.008	3	5.066e-3	2	NC	4	NC	1
530		min	0	3	-.077	2	-.006	2	-3.584e-3	3	1996.432	2	NC	1
531	19	max	0	1	.018	3	.007	3	4.065e-3	2	NC	1	NC	1
532		min	0	3	-.026	2	-.008	2	-2.903e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	3.793e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.21e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	8.102e-4	3	NC	1	NC	1
536		min	0	2	-.003	4	0	3	-4.78e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	1	1.241e-3	3	NC	1	NC	1
538		min	0	2	-.006	4	-.003	3	-9.039e-4	2	NC	1	NC	1
539	4	max	0	3	-.002	15	.006	1	1.672e-3	3	NC	3	NC	4
540		min	0	2	-.009	4	-.007	3	-1.33e-3	2	7304.134	4	5653.622	3
541	5	max	0	3	-.003	15	.009	1	2.103e-3	3	NC	5	NC	4
542		min	0	2	-.011	4	-.011	3	-1.756e-3	2	5699.49	4	3713.444	3
543	6	max	0	3	-.003	15	.013	1	2.534e-3	3	NC	5	NC	4
544		min	0	2	-.013	4	-.016	3	-2.182e-3	2	4796.723	4	2705.251	3
545	7	max	0	3	-.003	15	.017	2	2.964e-3	3	NC	5	NC	4
546		min	-.001	2	-.015	4	-.021	3	-2.607e-3	2	4253.828	4	2115.56	3
547	8	max	0	3	-.004	15	.021	2	3.395e-3	3	NC	5	NC	4
548		min	-.001	2	-.016	4	-.026	3	-3.033e-3	2	3928.009	4	1744.798	3
549	9	max	0	3	-.004	15	.024	2	3.826e-3	3	NC	5	NC	4
550		min	-.002	2	-.017	4	-.03	3	-3.459e-3	2	3752.634	4	1502.144	3
551	10	max	0	3	-.004	15	.027	2	4.257e-3	3	NC	5	NC	4
552		min	-.002	2	-.017	4	-.034	3	-3.885e-3	2	3697.154	4	1341.979	3
553	11	max	0	3	-.004	15	.029	1	4.688e-3	3	NC	5	NC	5
554		min	-.002	2	-.017	4	-.036	3	-4.311e-3	2	3752.634	4	1240.354	3
555	12	max	0	3	-.004	15	.03	1	5.119e-3	3	NC	5	NC	5
556		min	-.002	2	-.016	4	-.037	3	-4.737e-3	2	3928.009	4	1185.386	3
557	13	max	.001	3	-.004	15	.029	1	5.55e-3	3	NC	5	NC	5
558		min	-.002	2	-.015	4	-.036	3	-5.163e-3	2	4253.828	4	1173.633	3
559	14	max	.001	3	-.003	15	.027	1	5.981e-3	3	NC	5	NC	4
560		min	-.003	2	-.013	4	-.033	3	-5.589e-3	2	4796.723	4	1210.218	3
561	15	max	.001	3	-.003	15	.022	1	6.411e-3	3	NC	5	NC	4
562		min	-.003	2	-.011	4	-.028	3	-6.015e-3	2	5699.49	4	1313.95	3
563	16	max	.001	3	-.002	15	.016	1	6.842e-3	3	NC	3	NC	4
564		min	-.003	2	-.009	4	-.019	3	-6.441e-3	2	7304.134	4	1535.894	3



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Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	0	2	.007	1	7.273e-3	3	NC	1	NC	4
566		min	-.003	2	-.006	4	-.008	3	-6.866e-3	2	NC	1	2036.26	3
567	18	max	.001	3	.002	2	.007	3	7.704e-3	3	NC	1	NC	4
568		min	-.003	2	-.004	4	-.01	2	-7.292e-3	2	NC	1	3625.504	3
569	19	max	.002	3	.005	2	.026	3	8.135e-3	3	NC	1	NC	1
570		min	-.004	2	-.001	9	-.026	2	-7.718e-3	2	NC	1	NC	1
571	M16A	1	max	0	0	2	.008	3	2.35e-3	3	NC	1	NC	1
572		min	-.001	3	0	9	-.008	2	-2.355e-3	2	NC	1	NC	1
573	2	max	0	2	0	15	.001	3	2.257e-3	3	NC	1	NC	1
574		min	-.001	3	-.003	4	-.003	2	-2.247e-3	2	NC	1	NC	1
575	3	max	0	2	-.001	15	.004	1	2.165e-3	3	NC	1	NC	4
576		min	-.001	3	-.006	4	-.004	3	-2.14e-3	2	NC	1	5741.39	3
577	4	max	0	2	-.002	15	.007	1	2.072e-3	3	NC	3	NC	4
578		min	-.001	3	-.009	4	-.008	3	-2.033e-3	2	7304.134	4	4367.818	3
579	5	max	0	2	-.003	15	.009	1	1.979e-3	3	NC	5	NC	4
580		min	-.001	3	-.011	4	-.01	3	-1.926e-3	2	5699.49	4	3773.299	3
581	6	max	0	2	-.003	15	.01	1	1.887e-3	3	NC	5	NC	4
582		min	-.001	3	-.013	4	-.012	3	-1.819e-3	2	4796.723	4	3514.647	3
583	7	max	0	2	-.004	15	.011	1	1.794e-3	3	NC	5	NC	4
584		min	0	3	-.015	4	-.013	3	-1.711e-3	2	4253.828	4	3453.169	3
585	8	max	0	2	-.004	15	.011	1	1.701e-3	3	NC	5	NC	4
586		min	0	3	-.016	4	-.013	3	-1.604e-3	2	3928.009	4	3541.731	3
587	9	max	0	2	-.004	15	.01	1	1.609e-3	3	NC	5	NC	4
588		min	0	3	-.017	4	-.012	3	-1.497e-3	2	3752.634	4	3774.566	3
589	10	max	0	2	-.004	15	.009	1	1.516e-3	3	NC	5	NC	4
590		min	0	3	-.017	4	-.011	3	-1.39e-3	2	3697.154	4	4175.883	3
591	11	max	0	2	-.004	15	.008	1	1.424e-3	3	NC	5	NC	4
592		min	0	3	-.017	4	-.009	3	-1.283e-3	2	3752.634	4	4805.46	3
593	12	max	0	2	-.004	15	.006	1	1.331e-3	3	NC	5	NC	4
594		min	0	3	-.016	4	-.008	3	-1.175e-3	2	3928.009	4	5782.305	3
595	13	max	0	2	-.003	15	.005	1	1.238e-3	3	NC	5	NC	2
596		min	0	3	-.015	4	-.006	3	-1.068e-3	2	4253.828	4	7345.599	3
597	14	max	0	2	-.003	15	.003	1	1.146e-3	3	NC	5	NC	1
598		min	0	3	-.013	4	-.004	3	-9.609e-4	2	4796.723	4	NC	1
599	15	max	0	2	-.003	15	.002	1	1.053e-3	3	NC	5	NC	1
600		min	0	3	-.011	4	-.002	3	-8.536e-4	2	5699.49	4	NC	1
601	16	max	0	2	-.002	15	.001	9	9.603e-4	3	NC	3	NC	1
602		min	0	3	-.009	4	0	3	-7.464e-4	2	7304.134	4	NC	1
603	17	max	0	2	-.001	15	0	4	8.677e-4	3	NC	1	NC	1
604		min	0	3	-.006	4	0	2	-6.392e-4	2	NC	1	NC	1
605	18	max	0	2	0	15	0	3	7.751e-4	3	NC	1	NC	1
606		min	0	3	-.003	4	0	2	-5.32e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	6.824e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.248e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

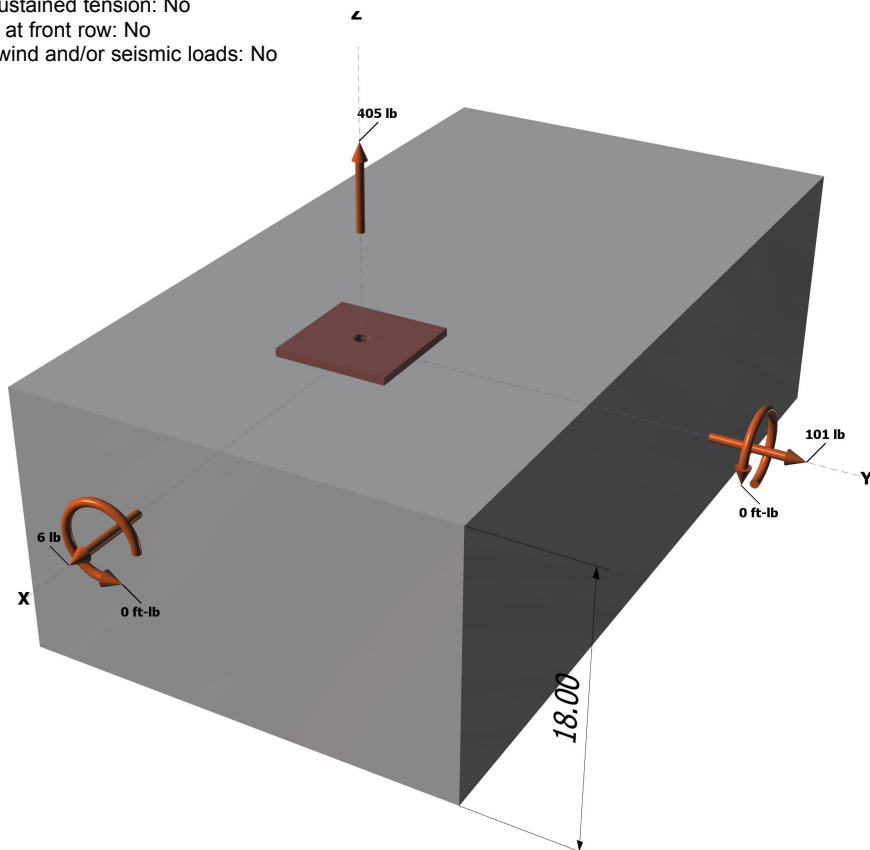
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

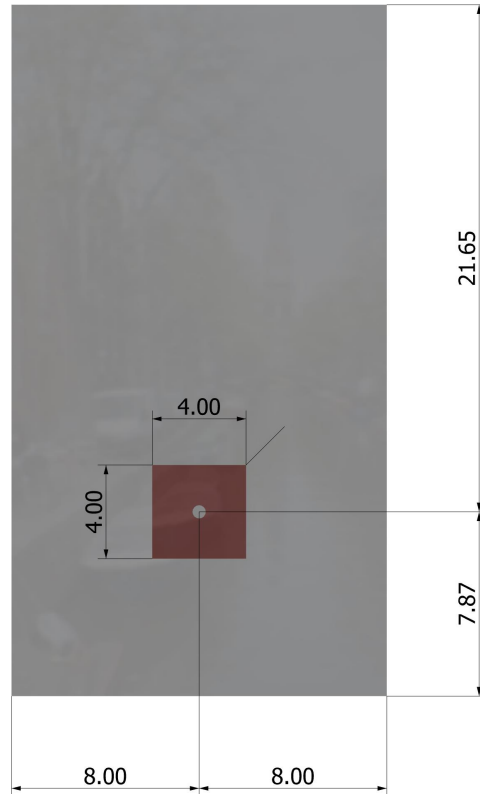
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
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Address:			
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E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

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Address:			
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

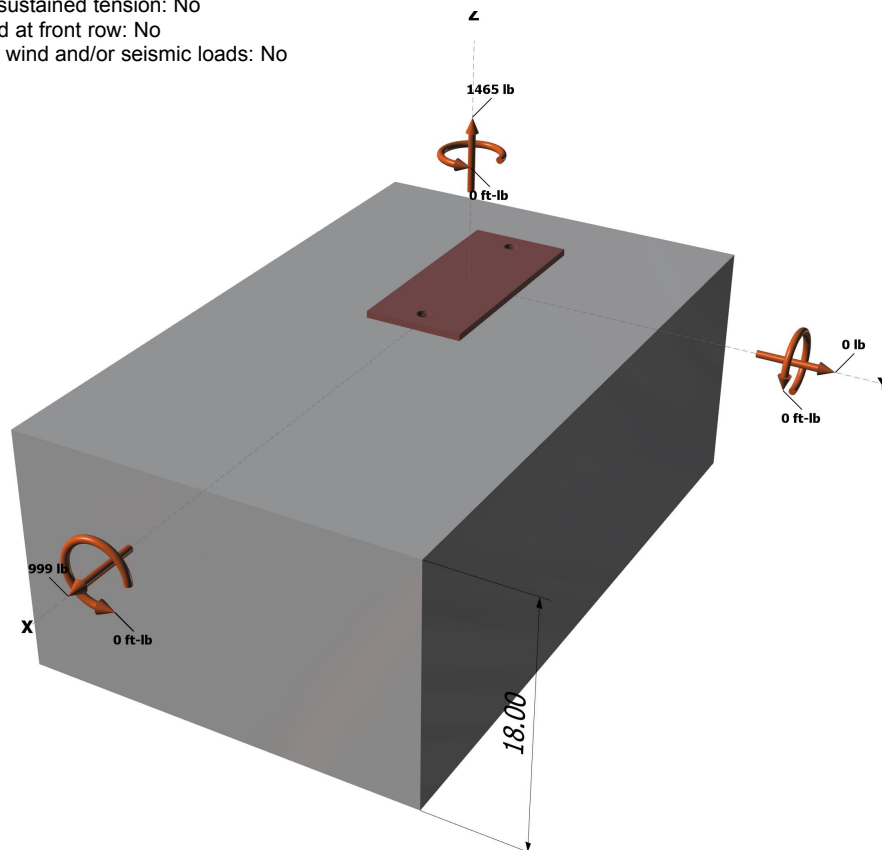
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



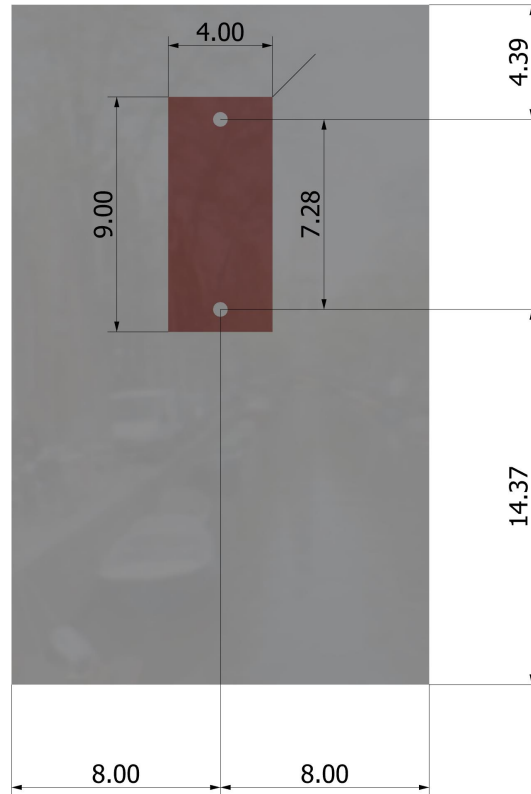
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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