

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.05	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	111 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.036 k-ft
$M_z$ =	0.219 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>92%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.367 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.555 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

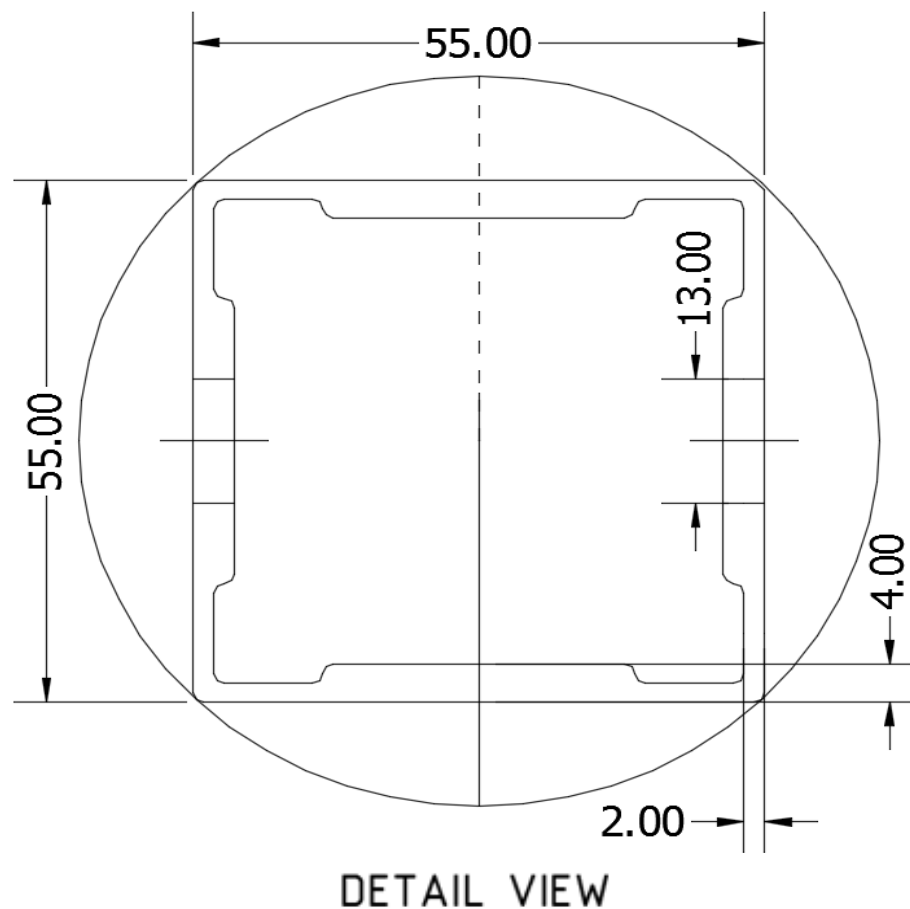
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.440 k-ft
$P_n$ =	0.637 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>34%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.526 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>21%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>48.30</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	18.93 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	-0.297 k-ft
$P_n$ =	0.644 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	18.592 k
Utilization =	<u>25%</u>



#### 5. FOUNDATION DESIGN CALCULATIONS

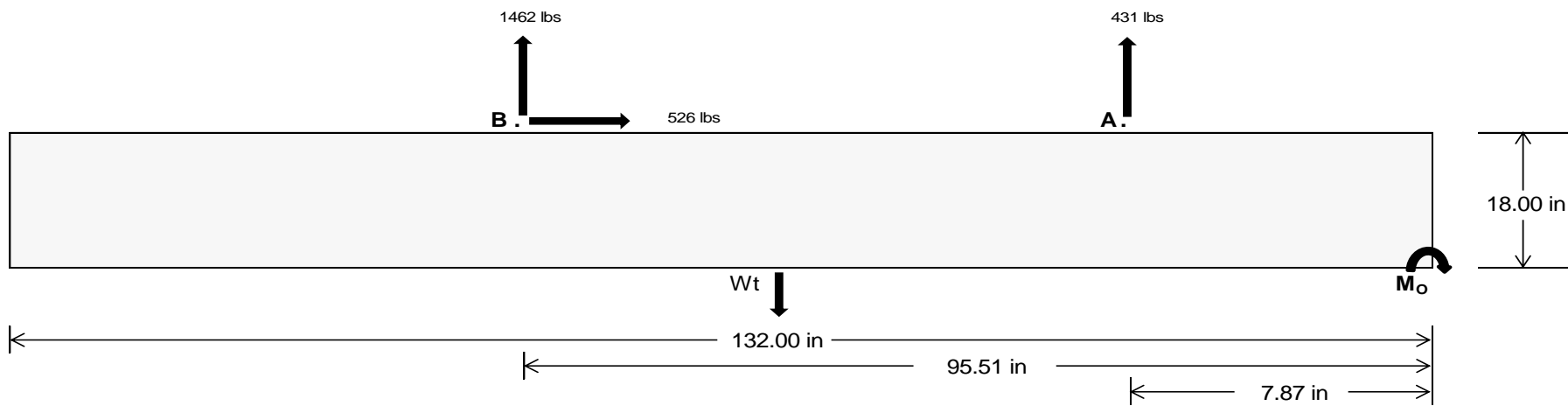
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>1887.60</u>	<u>6354.48</u> k
Compressive Load =	<u>4992.44</u>	<u>5173.80</u> k
Lateral Load =	<u>288.78</u>	<u>2280.41</u> k
Moment (Weak Axis) =	<u>0.59</u>	<u>0.38</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 152497.6$  in-lbs  
Resisting Force Required = 2310.57 lbs  
S.F. = 1.67  
Weight Required = 3850.95 lbs  
Minimum Width = **34 in** in  
Weight Provided = 6778.75 lbs

### Sliding

Force = 526.05 lbs  
Friction = 0.4  
Weight Required = 1315.12 lbs  
Resisting Weight = 6778.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 526.05 lbs  
Cohesion = 130 psf  
Area = 31.17 ft<sup>2</sup>  
Resisting = 3389.38 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 34in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 34in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 34in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.83 \text{ ft}) =$  **6779 lbs** **6978 lbs** **7178 lbs** **7377 lbs**

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	34 in	35 in	36 in	37 in	34 in	35 in	36 in	37 in	34 in	35 in	36 in	37 in	34 in	35 in	36 in	37 in
$F_A$	1594 lbs	1594 lbs	1594 lbs	1594 lbs	1903 lbs	1903 lbs	1903 lbs	1903 lbs	2503 lbs	2503 lbs	2503 lbs	2503 lbs	-862 lbs	-862 lbs	-862 lbs	-862 lbs
$F_B$	1649 lbs	1649 lbs	1649 lbs	1649 lbs	1972 lbs	1972 lbs	1972 lbs	1972 lbs	2593 lbs	2593 lbs	2593 lbs	2593 lbs	-2924 lbs	-2924 lbs	-2924 lbs	-2924 lbs
$F_V$	138 lbs	138 lbs	138 lbs	138 lbs	925 lbs	925 lbs	925 lbs	925 lbs	788 lbs	788 lbs	788 lbs	788 lbs	-1052 lbs	-1052 lbs	-1052 lbs	-1052 lbs
$P_{total}$	10021 lbs	10220 lbs	10420 lbs	10619 lbs	10654 lbs	10853 lbs	11052 lbs	11252 lbs	11875 lbs	12074 lbs	12273 lbs	12473 lbs	281 lbs	401 lbs	521 lbs	640 lbs
$M$	3873 lbs-ft	3873 lbs-ft	3873 lbs-ft	3873 lbs-ft	5755 lbs-ft	5755 lbs-ft	5755 lbs-ft	5755 lbs-ft	6931 lbs-ft	6931 lbs-ft	6931 lbs-ft	6931 lbs-ft	1437 lbs-ft	1437 lbs-ft	1437 lbs-ft	1437 lbs-ft
$e$	0.39 ft	0.38 ft	0.37 ft	0.36 ft	0.54 ft	0.53 ft	0.52 ft	0.51 ft	0.58 ft	0.57 ft	0.56 ft	0.56 ft	5.11 ft	3.58 ft	2.76 ft	2.24 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	253.8 psf	252.7 psf	251.7 psf	250.8 psf	241.1 psf	240.4 psf	239.8 psf	239.2 psf	259.7 psf	258.5 psf	257.4 psf	256.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	389.3 psf	384.4 psf	379.8 psf	375.4 psf	442.5 psf	436.1 psf	430.0 psf	424.3 psf	502.3 psf	494.2 psf	486.5 psf	479.2 psf	169.4 psf	47.8 psf	42.2 psf	42.5 psf

Maximum Bearing Pressure = 502 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 34in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

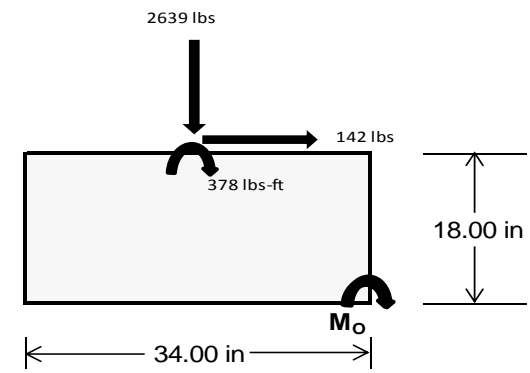
### Overturning Check

$M_O = 3147.4 \text{ ft-lbs}$   
 Resisting Force Required = 2221.70 lbs  
 S.F. = 1.67  
 Weight Required = 3702.83 lbs  
 Minimum Width = 34 in  
 Weight Provided = 6778.75 lbs

*A minimum 132in long x 34in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	34 in			34 in			34 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_Y$	239 lbs	599 lbs	209 lbs	886 lbs	2639 lbs	863 lbs	80 lbs	175 lbs	51 lbs
$F_V$	198 lbs	195 lbs	199 lbs	148 lbs	142 lbs	154 lbs	198 lbs	196 lbs	198 lbs
$P_{\text{total}}$	8631 lbs	8991 lbs	8601 lbs	8874 lbs	10628 lbs	8851 lbs	2534 lbs	2629 lbs	2505 lbs
$M$	788 lbs-ft	780 lbs-ft	792 lbs-ft	596 lbs-ft	592 lbs-ft	616 lbs-ft	785 lbs-ft	780 lbs-ft	786 lbs-ft
$e$	0.09 ft	0.09 ft	0.09 ft	0.07 ft	0.06 ft	0.07 ft	0.31 ft	0.30 ft	0.31 ft
$L/6$	0.47 ft	0.47 ft	0.47 ft	0.47 ft	0.47 ft	0.47 ft	0.47 ft	0.47 ft	0.47 ft
$f_{\text{min}}$	223.4 psf	235.5 psf	222.2 psf	244.2 psf	300.8 psf	242.2 psf	28.0 psf	31.4 psf	26.9 psf
$f_{\text{max}}$	330.4 psf	341.5 psf	329.8 psf	325.3 psf	381.2 psf	325.8 psf	134.6 psf	137.3 psf	133.8 psf



Maximum Bearing Pressure = 381 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 34in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 34in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

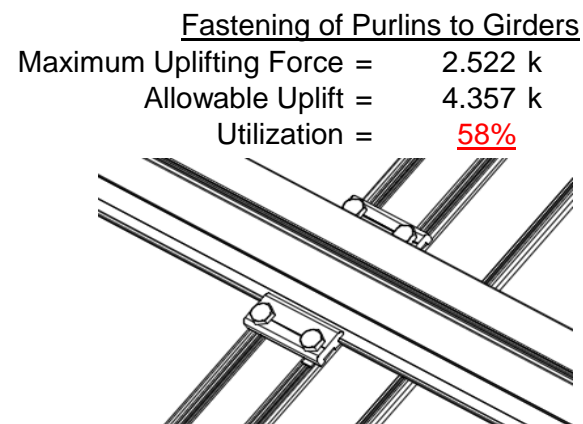
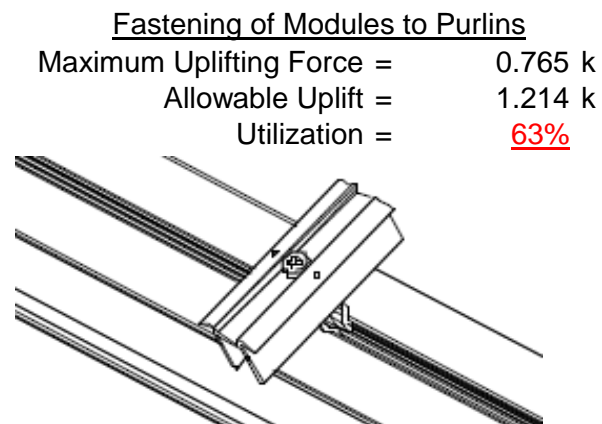
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

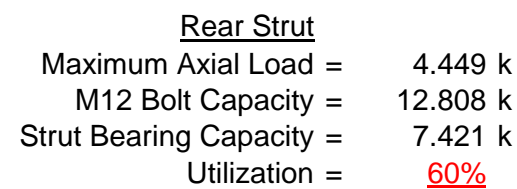
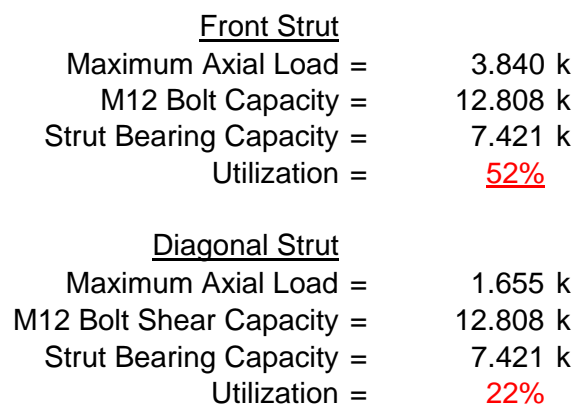
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



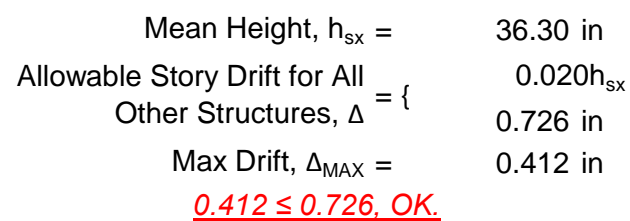
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 111 \text{ in}$$

$$J = 0.432$$

$$307.078$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.6 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 111$$

$$J = 0.432$$

$$195.283$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(lyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(lyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$R_b/t = 0.0$$

$$S_1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S_1 = 6.87$$

$$S_2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-98.517	-98.517	0	0
2	M14	y	-98.517	-98.517	0	0
3	M15	y	-157.628	-157.628	0	0
4	M16	y	-157.628	-157.628	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	226.59	226.59	0	0
2	M14	y	175.361	175.361	0	0
3	M15	y	98.517	98.517	0	0
4	M16	y	98.517	98.517	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 15° 150mph 30psf 9.25ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	70.944	1	1280.284	3	136.793	1	.005	14	.22	1	1.726	1
20		min	3.423	12	-904.031	1	-78.565	14	-.016	2	.004	12	-2.37	3
21	11	max	70.944	1	744.604	1	-2.902	12	.016	2	.095	1	.879	1
22		min	3.423	12	-1052.719	3	-107.64	1	0	3	0	12	-1.171	3
23	12	max	70.944	1	585.178	1	-1.951	12	.016	2	.032	4	.196	1
24		min	3.423	12	-825.155	3	-78.487	1	0	3	-.003	3	-.206	3
25	13	max	70.944	1	425.751	1	-1.001	12	.016	2	.015	5	.525	3
26		min	3.423	12	-597.591	3	-49.334	1	0	3	-.067	1	-.324	1
27	14	max	70.944	1	266.324	1	.022	3	.016	2	0	15	1.022	3
28		min	.47	15	-370.027	3	-20.181	1	0	3	-.102	1	-.68	1
29	15	max	70.944	1	106.897	1	8.972	1	.016	2	-.003	12	1.285	3
30		min	-9.757	5	-142.462	3	-12.793	5	0	3	-.108	1	-.871	1
31	16	max	70.944	1	85.102	3	38.124	1	.016	2	-.002	12	1.315	3
32		min	-20.307	5	-52.529	1	-11.323	5	0	3	-.084	1	-.899	1
33	17	max	70.944	1	312.666	3	67.277	1	.016	2	0	3	1.11	3
34		min	-30.858	5	-211.956	1	-9.852	5	0	3	-.045	4	-.763	1
35	18	max	70.944	1	540.23	3	96.43	1	.016	2	.054	1	.672	3
36		min	-41.408	5	-371.383	1	-8.382	5	0	3	-.047	5	-.464	1
37	19	max	70.944	1	767.795	3	125.583	1	.016	2	.168	1	0	1
38		min	-51.958	5	-530.81	1	-6.912	5	0	3	-.055	5	0	3
39	M14	1	max	55.314	4	569.461	1	-4.833	12	.01	.194	1	0	1
40		min	1.482	12	-611.948	3	-129.726	1	-.013	1	.009	12	0	3
41	2	max	44.764	4	410.034	1	-3.883	12	.01	3	.095	4	.539	3
42		min	1.482	12	-437.036	3	-100.573	1	-.013	1	.005	12	-.503	1
43	3	max	34.698	1	250.607	1	-2.932	12	.01	3	.052	5	.898	3
44		min	1.482	12	-262.124	3	-71.421	1	-.013	1	-.013	1	-.843	1
45	4	max	34.698	1	91.18	1	-1.982	12	.01	3	.028	5	1.078	3
46		min	1.482	12	-87.212	3	-42.268	1	-.013	1	-.071	1	-1.018	1
47	5	max	34.698	1	87.701	3	-.792	10	.01	3	.006	5	1.078	3
48		min	1.482	12	-68.246	1	-24.146	4	-.013	1	-.1	1	-1.03	1
49	6	max	34.698	1	262.613	3	16.038	1	.01	3	-.004	12	.898	3
50		min	-4.518	5	-227.673	1	-19.535	5	-.013	1	-.098	1	-.878	1
51	7	max	34.698	1	437.525	3	45.191	1	.01	3	-.003	12	.538	3
52		min	-15.068	5	-387.1	1	-18.065	5	-.013	1	-.067	1	-.562	1
53	8	max	34.698	1	612.437	3	74.344	1	.01	3	0	10	-.001	15
54		min	-25.619	5	-546.527	1	-16.595	5	-.013	1	-.054	4	-.094	2
55	9	max	34.698	1	787.349	3	103.497	1	.01	3	.086	1	.561	1
56		min	-36.169	5	-705.954	1	-15.124	5	-.013	1	-.068	5	-.721	3
57	10	max	56.983	4	962.262	3	132.65	1	.01	3	.207	1	1.369	1
58		min	1.482	12	-865.38	1	-80.15	14	-.013	1	.004	12	-1.62	3
59	11	max	46.432	4	705.954	1	-2.77	12	.013	1	.095	4	.561	1
60		min	1.482	12	-787.349	3	-103.497	1	-.01	3	0	3	-.721	3
61	12	max	35.882	4	546.527	1	-1.819	12	.013	1	.051	5	-.001	15
62		min	1.482	12	-612.437	3	-74.344	1	-.01	3	-.005	1	-.094	2
63	13	max	34.698	1	387.1	1	-.869	12	.013	1	.027	5	.538	3
64		min	1.482	12	-437.525	3	-45.191	1	-.01	3	-.067	1	-.562	1
65	14	max	34.698	1	227.673	1	.219	3	.013	1	.005	5	.898	3
66		min	1.482	12	-262.613	3	-24.702	4	-.01	3	-.098	1	-.878	1
67	15	max	34.698	1	68.246	1	13.115	1	.013	1	-.003	12	1.078	3
68		min	-2.676	5	-87.701	3	-19.648	5	-.01	3	-.1	1	-1.03	1
69	16	max	34.698	1	87.212	3	42.268	1	.013	1	-.001	12	1.078	3
70		min	-13.226	5	-91.18	1	-18.177	5	-.01	3	-.071	1	-1.018	1
71	17	max	34.698	1	262.124	3	71.421	1	.013	1	.002	3	.898	3
72		min	-23.776	5	-250.607	1	-16.707	5	-.01	3	-.057	4	-.843	1
73	18	max	34.698	1	437.036	3	100.573	1	.013	1	.075	1	.539	3
74		min	-34.327	5	-410.034	1	-15.237	5	-.01	3	-.07	5	-.503	1
75	19	max	34.698	1	611.948	3	129.726	1	.013	1	.194	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-44.877	5	-569.461	1	-13.767	5	-.01	3	-.085	5	0	3
77	M15	1	max	73.517	5	719.856	2	-4.787	12	.013	1	.194	1	0	2
78			min	-36.253	1	-345.747	3	-129.72	1	-.009	3	.009	12	0	3
79		2	max	62.967	5	515.766	2	-3.837	12	.013	1	.13	4	.306	3
80			min	-36.253	1	-249.814	3	-100.567	1	-.009	3	.004	12	-.635	2
81		3	max	52.416	5	311.676	2	-2.886	12	.013	1	.078	5	.514	3
82			min	-36.253	1	-153.88	3	-71.414	1	-.009	3	-.013	1	-1.06	2
83		4	max	41.866	5	107.585	2	-1.936	12	.013	1	.044	5	.622	3
84			min	-36.253	1	-57.946	3	-42.261	1	-.009	3	-.071	1	-1.276	2
85		5	max	31.316	5	37.987	3	-.814	10	.013	1	.012	5	.633	3
86			min	-36.253	1	-96.505	2	-33.676	4	-.009	3	-.1	1	-1.281	2
87		6	max	20.765	5	133.921	3	16.044	1	.013	1	-.004	12	.544	3
88			min	-36.253	1	-300.596	2	-29.062	5	-.009	3	-.098	1	-1.077	2
89		7	max	10.215	5	229.855	3	45.197	1	.013	1	-.003	12	.357	3
90			min	-36.253	1	-504.686	2	-27.592	5	-.009	3	-.067	1	-.663	2
91		8	max	-.186	15	325.788	3	74.35	1	.013	1	0	10	.072	3
92			min	-36.253	1	-708.777	2	-26.121	5	-.009	3	-.078	4	-.055	1
93		9	max	-1.808	12	421.722	3	103.503	1	.013	1	.086	1	.793	2
94			min	-36.253	1	-912.867	2	-24.651	5	-.009	3	-.102	5	-.312	3
95		10	max	-1.808	12	517.656	3	132.656	1	.013	1	.207	1	1.837	2
96			min	-36.253	1	-1116.957	2	-84.981	14	-.009	3	.004	12	-.795	3
97		11	max	7.032	5	912.867	2	-2.816	12	.009	3	.129	4	.793	2
98			min	-36.253	1	-421.722	3	-103.503	1	-.013	1	0	12	-.312	3
99		12	max	-1.808	12	708.777	2	-1.865	12	.009	3	.075	5	.072	3
100			min	-36.253	1	-325.788	3	-74.35	1	-.013	1	-.005	1	-.055	1
101		13	max	-1.808	12	504.686	2	-.915	12	.009	3	.041	5	.357	3
102			min	-36.253	1	-229.855	3	-45.197	1	-.013	1	-.067	1	-.663	2
103		14	max	-1.808	12	300.596	2	.143	3	.009	3	.009	5	.544	3
104			min	-36.253	1	-133.921	3	-34.244	4	-.013	1	-.098	1	-1.077	2
105		15	max	-1.808	12	96.505	2	13.108	1	.009	3	-.003	12	.633	3
106			min	-42.939	4	-37.987	3	-29.175	5	-.013	1	-.1	1	-1.281	2
107		16	max	-1.808	12	57.946	3	42.261	1	.009	3	-.001	12	.622	3
108			min	-53.49	4	-107.585	2	-27.705	5	-.013	1	-.071	1	-1.276	2
109		17	max	-1.808	12	153.88	3	71.414	1	.009	3	.002	3	.514	3
110			min	-64.04	4	-311.676	2	-26.235	5	-.013	1	-.082	4	-1.06	2
111		18	max	-1.808	12	249.814	3	100.567	1	.009	3	.075	1	.306	3
112			min	-74.59	4	-515.766	2	-24.764	5	-.013	1	-.105	5	-.635	2
113		19	max	-1.808	12	345.747	3	129.72	1	.009	3	.194	1	0	2
114			min	-85.141	4	-719.856	2	-23.294	5	-.013	1	-.13	5	0	5
115	M16	1	max	73.198	5	682.899	2	-4.546	12	.013	1	.169	1	0	2
116			min	-75.004	1	-317.811	3	-125.794	1	-.012	3	.007	12	0	3
117		2	max	62.648	5	478.809	2	-3.595	12	.013	1	.095	4	.277	3
118			min	-75.004	1	-221.877	3	-96.641	1	-.012	3	.003	12	-.597	2
119		3	max	52.097	5	274.719	2	-2.645	12	.013	1	.056	5	.456	3
120			min	-75.004	1	-125.943	3	-67.488	1	-.012	3	-.029	1	-.984	2
121		4	max	41.547	5	70.628	2	-1.695	12	.013	1	.032	5	.536	3
122			min	-75.004	1	-30.01	3	-38.335	1	-.012	3	-.084	1	-1.162	2
123		5	max	30.996	5	65.924	3	-.525	10	.013	1	.009	5	.518	3
124			min	-75.004	1	-133.462	2	-23.786	4	-.012	3	-.108	1	-1.129	2
125		6	max	20.446	5	161.858	3	19.971	1	.013	1	-.004	12	.401	3
126			min	-75.004	1	-337.553	2	-20.099	5	-.012	3	-.102	1	-.887	2
127		7	max	9.896	5	257.791	3	49.124	1	.013	1	-.003	12	.185	3
128			min	-75.004	1	-541.643	2	-18.629	5	-.012	3	-.067	1	-.436	2
129		8	max	-.388	15	353.725	3	78.276	1	.013	1	0	10	.226	2
130			min	-75.004	1	-745.734	2	-17.159	5	-.012	3	-.052	4	-.129	3
131		9	max	-3.349	12	449.659	3	107.429	1	.013	1	.094	1	1.097	2
132			min	-75.004	1	-949.824	2	-15.688	5	-.012	3	-.068	5	-.542	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-3.349	12	545.593	3	136.582	1	.013	1	.219	1	2.178	2
134		min	-75.004	1	-1153.914	2	-82.493	14	-.012	3	.005	12	-1.053	3
135	11	max	3.634	5	949.824	2	-3.057	12	.012	3	.097	4	1.097	2
136		min	-75.004	1	-449.659	3	-107.429	1	-.013	1	.001	12	-.542	3
137	12	max	-3.349	12	745.734	2	-2.107	12	.012	3	.051	4	.226	2
138		min	-75.004	1	-353.725	3	-78.276	1	-.013	1	-.002	3	-.129	3
139	13	max	-3.349	12	541.643	2	-1.156	12	.012	3	.025	5	.185	3
140		min	-75.004	1	-257.791	3	-49.124	1	-.013	1	-.067	1	-.436	2
141	14	max	-3.349	12	337.553	2	-.206	12	.012	3	.002	5	.401	3
142		min	-75.004	1	-161.858	3	-26.351	4	-.013	1	-.102	1	-.887	2
143	15	max	-3.349	12	133.462	2	9.182	1	.012	3	-.003	12	.518	3
144		min	-75.004	1	-65.924	3	-20.62	5	-.013	1	-.108	1	-1.129	2
145	16	max	-3.349	12	30.01	3	38.335	1	.012	3	-.002	12	.536	3
146		min	-75.004	1	-70.628	2	-19.15	5	-.013	1	-.084	1	-1.162	2
147	17	max	-3.349	12	125.943	3	67.488	1	.012	3	0	3	.456	3
148		min	-75.481	4	-274.719	2	-17.679	5	-.013	1	-.066	4	-.984	2
149	18	max	-3.349	12	221.877	3	96.641	1	.012	3	.055	1	.277	3
150		min	-86.031	4	-478.809	2	-16.209	5	-.013	1	-.077	5	-.597	2
151	19	max	-3.349	12	317.811	3	125.794	1	.012	3	.169	1	0	2
152		min	-96.581	4	-682.899	2	-14.739	5	-.013	1	-.093	5	0	5
153	M2	1	max	1167.571	1	2.335	4	1.012	1	0	0	3	0	1
154		min	-1367.164	3	.573	15	-71.214	4	0	4	0	1	0	1
155	2	max	1167.9	1	2.32	4	1.012	1	0	3	0	1	0	15
156		min	-1366.918	3	.569	15	-71.499	4	0	4	-.016	4	0	4
157	3	max	1168.228	1	2.305	4	1.012	1	0	3	0	1	0	15
158		min	-1366.671	3	.566	15	-71.784	4	0	4	-.032	4	-.001	4
159	4	max	1168.556	1	2.29	4	1.012	1	0	3	0	1	0	15
160		min	-1366.425	3	.562	15	-72.068	4	0	4	-.048	4	-.002	4
161	5	max	1168.885	1	2.274	4	1.012	1	0	3	0	1	0	15
162		min	-1366.179	3	.559	15	-72.353	4	0	4	-.064	4	-.002	4
163	6	max	1169.213	1	2.259	4	1.012	1	0	3	.001	1	0	15
164		min	-1365.932	3	.555	15	-72.638	4	0	4	-.08	4	-.003	4
165	7	max	1169.542	1	2.244	4	1.012	1	0	3	.001	1	0	15
166		min	-1365.686	3	.552	15	-72.923	4	0	4	-.096	4	-.003	4
167	8	max	1169.87	1	2.229	4	1.012	1	0	3	.002	1	0	15
168		min	-1365.44	3	.548	15	-73.208	4	0	4	-.112	4	-.004	4
169	9	max	1170.199	1	2.213	4	1.012	1	0	3	.002	1	0	15
170		min	-1365.193	3	.544	15	-73.493	4	0	4	-.128	4	-.004	4
171	10	max	1170.527	1	2.198	4	1.012	1	0	3	.002	1	-.001	15
172		min	-1364.947	3	.541	15	-73.777	4	0	4	-.145	4	-.005	4
173	11	max	1170.855	1	2.183	4	1.012	1	0	3	.002	1	-.001	15
174		min	-1364.701	3	.537	15	-74.062	4	0	4	-.161	4	-.005	4
175	12	max	1171.184	1	2.168	4	1.012	1	0	3	.002	1	-.001	15
176		min	-1364.454	3	.534	15	-74.347	4	0	4	-.177	4	-.005	4
177	13	max	1171.512	1	2.152	4	1.012	1	0	3	.003	1	-.001	15
178		min	-1364.208	3	.53	15	-74.632	4	0	4	-.194	4	-.006	4
179	14	max	1171.841	1	2.137	4	1.012	1	0	3	.003	1	-.002	15
180		min	-1363.962	3	.526	15	-74.917	4	0	4	-.21	4	-.006	4
181	15	max	1172.169	1	2.122	4	1.012	1	0	3	.003	1	-.002	15
182		min	-1363.715	3	.523	15	-75.202	4	0	4	-.227	4	-.007	4
183	16	max	1172.498	1	2.107	4	1.012	1	0	3	.003	1	-.002	15
184		min	-1363.469	3	.519	15	-75.486	4	0	4	-.244	4	-.007	4
185	17	max	1172.826	1	2.091	4	1.012	1	0	3	.004	1	-.002	15
186		min	-1363.223	3	.516	15	-75.771	4	0	4	-.26	4	-.008	4
187	18	max	1173.155	1	2.076	4	1.012	1	0	3	.004	1	-.002	15
188		min	-1362.976	3	.512	15	-76.056	4	0	4	-.277	4	-.008	4
189	19	max	1173.483	1	2.061	4	1.012	1	0	3	.004	1	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1362.73	3	.509	15	-76.341	4	0	4	-.294	4	-.009	4
191	M3	1	max	406.878	2	8.107	4	.017	1	0	3	0	1	.009	4
192			min	-526.338	3	1.918	15	-1.165	5	0	4	-.011	4	.002	15
193		2	max	406.708	2	7.334	4	.017	1	0	3	0	1	.006	4
194			min	-526.466	3	1.737	15	-.623	5	0	4	-.011	4	.001	12
195		3	max	406.538	2	6.562	4	.024	14	0	3	0	1	.003	2
196			min	-526.593	3	1.555	15	-.081	5	0	4	-.012	4	0	3
197		4	max	406.367	2	5.789	4	.513	4	0	3	0	1	0	2
198			min	-526.721	3	1.374	15	.001	12	0	4	-.011	4	-.002	3
199		5	max	406.197	2	5.017	4	1.055	4	0	3	0	1	0	15
200			min	-526.849	3	1.192	15	.001	12	0	4	-.011	4	-.003	3
201		6	max	406.027	2	4.245	4	1.597	4	0	3	0	1	0	15
202			min	-526.977	3	1.01	15	.001	12	0	4	-.011	4	-.004	6
203		7	max	405.856	2	3.472	4	2.139	4	0	3	0	1	-.001	15
204			min	-527.104	3	.829	15	.001	12	0	4	-.01	4	-.006	6
205		8	max	405.686	2	2.7	4	2.681	4	0	3	0	1	-.002	15
206			min	-527.232	3	.647	15	.001	12	0	4	-.009	5	-.007	6
207		9	max	405.516	2	1.927	4	3.224	4	0	3	0	1	-.002	15
208			min	-527.36	3	.466	15	.001	12	0	4	-.008	5	-.008	6
209		10	max	405.345	2	1.155	4	3.766	4	0	3	0	1	-.002	15
210			min	-527.488	3	.284	15	.001	12	0	4	-.006	5	-.009	6
211		11	max	405.175	2	.446	2	4.308	4	0	3	0	1	-.002	15
212			min	-527.615	3	-.026	3	.001	12	0	4	-.004	5	-.009	6
213		12	max	405.005	2	-.079	15	4.85	4	0	3	0	1	-.002	15
214			min	-527.743	3	-.478	3	.001	12	0	4	-.002	5	-.009	6
215		13	max	404.834	2	-.261	15	5.392	4	0	3	0	1	-.002	15
216			min	-527.871	3	-1.164	6	.001	12	0	4	0	5	-.009	6
217		14	max	404.664	2	-.442	15	5.934	4	0	3	.002	4	-.002	15
218			min	-527.999	3	-1.936	6	.001	12	0	4	0	12	-.008	6
219		15	max	404.494	2	-.624	15	6.476	4	0	3	.005	4	-.002	15
220			min	-528.126	3	-2.708	6	.001	12	0	4	0	12	-.007	6
221		16	max	404.323	2	-.805	15	7.018	4	0	3	.008	4	-.001	15
222			min	-528.254	3	-3.481	6	.001	12	0	4	0	12	-.006	6
223		17	max	404.153	2	-.987	15	7.561	4	0	3	.011	4	0	15
224			min	-528.382	3	-4.253	6	.001	12	0	4	0	12	-.004	6
225		18	max	403.983	2	-1.168	15	8.103	4	0	3	.014	4	0	15
226			min	-528.51	3	-5.026	6	.001	12	0	4	0	12	-.002	6
227		19	max	403.812	2	-1.35	15	8.645	4	0	3	.018	4	0	1
228			min	-528.637	3	-5.798	6	.001	12	0	4	0	12	0	1
229	M4	1	max	1296.071	1	0	1	-.299	12	0	1	.009	4	0	1
230			min	-433.167	3	0	1	-220.792	4	0	1	0	10	0	1
231		2	max	1296.241	1	0	1	-.299	12	0	1	0	12	0	1
232			min	-433.039	3	0	1	-220.939	4	0	1	-.016	4	0	1
233		3	max	1296.412	1	0	1	-.299	12	0	1	0	12	0	1
234			min	-432.912	3	0	1	-221.087	4	0	1	-.041	4	0	1
235		4	max	1296.582	1	0	1	-.299	12	0	1	0	12	0	1
236			min	-432.784	3	0	1	-221.234	4	0	1	-.067	4	0	1
237		5	max	1296.752	1	0	1	-.299	12	0	1	0	12	0	1
238			min	-432.656	3	0	1	-221.382	4	0	1	-.092	4	0	1
239		6	max	1296.923	1	0	1	-.299	12	0	1	0	12	0	1
240			min	-432.528	3	0	1	-221.53	4	0	1	-.118	4	0	1
241		7	max	1297.093	1	0	1	-.299	12	0	1	0	12	0	1
242			min	-432.401	3	0	1	-221.677	4	0	1	-.143	4	0	1
243		8	max	1297.263	1	0	1	-.299	12	0	1	0	12	0	1
244			min	-432.273	3	0	1	-221.825	4	0	1	-.169	4	0	1
245		9	max	1297.434	1	0	1	-.299	12	0	1	0	12	0	1
246			min	-432.145	3	0	1	-221.973	4	0	1	-.194	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1297.604	1	0	1	-.299	12	0	1	0	12	0	1
248		min	-432.017	3	0	1	-222.12	4	0	1	-.22	4	0	1
249	11	max	1297.775	1	0	1	-.299	12	0	1	0	12	0	1
250		min	-431.89	3	0	1	-222.268	4	0	1	-.245	4	0	1
251	12	max	1297.945	1	0	1	-.299	12	0	1	0	12	0	1
252		min	-431.762	3	0	1	-222.415	4	0	1	-.271	4	0	1
253	13	max	1298.115	1	0	1	-.299	12	0	1	0	12	0	1
254		min	-431.634	3	0	1	-222.563	4	0	1	-.296	4	0	1
255	14	max	1298.286	1	0	1	-.299	12	0	1	0	12	0	1
256		min	-431.506	3	0	1	-222.711	4	0	1	-.322	4	0	1
257	15	max	1298.456	1	0	1	-.299	12	0	1	0	12	0	1
258		min	-431.379	3	0	1	-222.858	4	0	1	-.347	4	0	1
259	16	max	1298.626	1	0	1	-.299	12	0	1	0	12	0	1
260		min	-431.251	3	0	1	-223.006	4	0	1	-.373	4	0	1
261	17	max	1298.797	1	0	1	-.299	12	0	1	0	12	0	1
262		min	-431.123	3	0	1	-223.154	4	0	1	-.399	4	0	1
263	18	max	1298.967	1	0	1	-.299	12	0	1	0	12	0	1
264		min	-430.995	3	0	1	-223.301	4	0	1	-.424	4	0	1
265	19	max	1299.137	1	0	1	-.299	12	0	1	0	12	0	1
266		min	-430.868	3	0	1	-223.449	4	0	1	-.45	4	0	1
267	M6	1	max	3734.218	1	2.939	2	0	1	0	0	4	0	1
268		min	-4449.348	3	-.085	3	-71.83	4	0	4	0	1	0	1
269	2	max	3734.547	1	2.928	2	0	1	0	1	0	1	0	3
270		min	-4449.102	3	-.094	3	-72.115	4	0	4	-.016	4	0	2
271	3	max	3734.875	1	2.916	2	0	1	0	1	0	1	0	3
272		min	-4448.856	3	-.103	3	-72.4	4	0	4	-.032	4	-.001	2
273	4	max	3735.204	1	2.904	2	0	1	0	1	0	1	0	3
274		min	-4448.609	3	-.112	3	-72.685	4	0	4	-.048	4	-.002	2
275	5	max	3735.532	1	2.892	2	0	1	0	1	0	1	0	3
276		min	-4448.363	3	-.12	3	-72.97	4	0	4	-.064	4	-.003	2
277	6	max	3735.86	1	2.88	2	0	1	0	1	0	1	0	3
278		min	-4448.117	3	-.129	3	-73.255	4	0	4	-.08	4	-.003	2
279	7	max	3736.189	1	2.868	2	0	1	0	1	0	1	0	3
280		min	-4447.87	3	-.138	3	-73.539	4	0	4	-.097	4	-.004	2
281	8	max	3736.517	1	2.856	2	0	1	0	1	0	1	0	3
282		min	-4447.624	3	-.147	3	-73.824	4	0	4	-.113	4	-.004	2
283	9	max	3736.846	1	2.844	2	0	1	0	1	0	1	0	3
284		min	-4447.378	3	-.156	3	-74.109	4	0	4	-.129	4	-.005	2
285	10	max	3737.174	1	2.832	2	0	1	0	1	0	1	0	3
286		min	-4447.132	3	-.165	3	-74.394	4	0	4	-.146	4	-.006	2
287	11	max	3737.503	1	2.821	2	0	1	0	1	0	1	0	3
288		min	-4446.885	3	-.174	3	-74.679	4	0	4	-.162	4	-.006	2
289	12	max	3737.831	1	2.809	2	0	1	0	1	0	1	0	3
290		min	-4446.639	3	-.183	3	-74.964	4	0	4	-.179	4	-.007	2
291	13	max	3738.159	1	2.797	2	0	1	0	1	0	1	0	3
292		min	-4446.393	3	-.192	3	-75.248	4	0	4	-.195	4	-.008	2
293	14	max	3738.488	1	2.785	2	0	1	0	1	0	1	0	3
294		min	-4446.146	3	-.201	3	-75.533	4	0	4	-.212	4	-.008	2
295	15	max	3738.816	1	2.773	2	0	1	0	1	0	1	0	3
296		min	-4445.9	3	-.21	3	-75.818	4	0	4	-.229	4	-.009	2
297	16	max	3739.145	1	2.761	2	0	1	0	1	0	1	0	3
298		min	-4445.654	3	-.219	3	-76.103	4	0	4	-.246	4	-.009	2
299	17	max	3739.473	1	2.749	2	0	1	0	1	0	1	0	3
300		min	-4445.407	3	-.228	3	-76.388	4	0	4	-.263	4	-.01	2
301	18	max	3739.802	1	2.737	2	0	1	0	1	0	1	0	3
302		min	-4445.161	3	-.236	3	-76.673	4	0	4	-.28	4	-.011	2
303	19	max	3740.13	1	2.725	2	0	1	0	1	0	1	0	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-4444.915	3	-.245	3	-76.957	4	0	4	-.297	4	-.011	2
305	M7	1	max	1526.181	2	8.115	6	0	1	0	0	1	.011	2
306		min	-1652.367	3	1.904	15	-1.235	5	0	4	-.011	4	0	3
307		2	max	1526.01	2	7.342	6	0	1	0	0	1	.009	2
308		min	-1652.495	3	1.723	15	-.693	5	0	4	-.012	4	-.002	3
309		3	max	1525.84	2	6.57	6	0	1	0	0	1	.006	2
310		min	-1652.623	3	1.541	15	-.151	5	0	4	-.012	4	-.004	3
311		4	max	1525.67	2	5.798	6	.435	4	0	0	1	.004	2
312		min	-1652.75	3	1.36	15	0	1	0	4	-.012	4	-.005	3
313		5	max	1525.499	2	5.025	6	.977	4	0	0	1	.002	2
314		min	-1652.878	3	1.178	15	0	1	0	4	-.011	4	-.006	3
315		6	max	1525.329	2	4.253	6	1.519	4	0	0	1	0	2
316		min	-1653.006	3	.996	15	0	1	0	4	-.011	4	-.007	3
317		7	max	1525.159	2	3.48	6	2.061	4	0	0	1	-.001	15
318		min	-1653.134	3	.815	15	0	1	0	4	-.01	4	-.007	3
319		8	max	1524.988	2	2.708	6	2.603	4	0	0	1	-.002	15
320		min	-1653.262	3	.594	12	0	1	0	4	-.009	4	-.008	3
321		9	max	1524.818	2	2.09	2	3.145	4	0	0	1	-.002	15
322		min	-1653.389	3	.293	12	0	1	0	4	-.008	4	-.008	4
323		10	max	1524.648	2	1.488	2	3.687	4	0	0	1	-.002	15
324		min	-1653.517	3	-.087	3	0	1	0	4	-.006	4	-.009	4
325		11	max	1524.477	2	.886	2	4.229	4	0	0	1	-.002	15
326		min	-1653.645	3	-.538	3	0	1	0	4	-.005	4	-.009	4
327		12	max	1524.307	2	.284	2	4.772	4	0	0	1	-.002	15
328		min	-1653.773	3	-.99	3	0	1	0	4	-.003	5	-.009	4
329		13	max	1524.137	2	-.275	15	5.314	4	0	0	1	-.002	15
330		min	-1653.9	3	-1.441	3	0	1	0	4	0	5	-.009	4
331		14	max	1523.966	2	-.456	15	5.856	4	0	0	1	-.002	15
332		min	-1654.028	3	-1.927	4	0	1	0	4	0	1	-.008	4
333		15	max	1523.796	2	-.638	15	6.398	4	0	0	1	-.002	15
334		min	-1654.156	3	-2.699	4	0	1	0	4	0	1	-.007	4
335		16	max	1523.626	2	-.819	15	6.94	4	0	0	1	-.001	15
336		min	-1654.284	3	-3.472	4	0	1	0	4	0	1	-.006	4
337		17	max	1523.455	2	-1.001	15	7.482	4	0	0	1	0	15
338		min	-1654.411	3	-4.244	4	0	1	0	4	0	1	-.004	4
339		18	max	1523.285	2	-1.182	15	8.024	4	0	0	1	0	15
340		min	-1654.539	3	-5.016	4	0	1	0	4	0	1	-.002	4
341		19	max	1523.115	2	-1.364	15	8.566	4	0	0	1	0	1
342		min	-1654.667	3	-5.789	4	0	1	0	4	0	1	0	1
343	M8	1	max	3837.269	1	0	1	0	1	0	0	1	0	1
344		min	-1454.297	3	0	1	-216.015	4	0	1	0	1	0	1
345		2	max	3837.44	1	0	1	0	1	0	0	1	0	1
346		min	-1454.169	3	0	1	-216.162	4	0	1	-.016	4	0	1
347		3	max	3837.61	1	0	1	0	1	0	0	1	0	1
348		min	-1454.041	3	0	1	-216.31	4	0	1	-.041	4	0	1
349		4	max	3837.78	1	0	1	0	1	0	0	1	0	1
350		min	-1453.914	3	0	1	-216.457	4	0	1	-.066	4	0	1
351		5	max	3837.951	1	0	1	0	1	0	0	1	0	1
352		min	-1453.786	3	0	1	-216.605	4	0	1	-.09	4	0	1
353		6	max	3838.121	1	0	1	0	1	0	0	1	0	1
354		min	-1453.658	3	0	1	-216.753	4	0	1	-.115	4	0	1
355		7	max	3838.291	1	0	1	0	1	0	0	1	0	1
356		min	-1453.53	3	0	1	-216.9	4	0	1	-.14	4	0	1
357		8	max	3838.462	1	0	1	0	1	0	0	1	0	1
358		min	-1453.403	3	0	1	-217.048	4	0	1	-.165	4	0	1
359		9	max	3838.632	1	0	1	0	1	0	0	1	0	1
360		min	-1453.275	3	0	1	-217.196	4	0	1	-.19	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3838.802	1	0	1	0	1	0	1	0	1	0	1
362			min	-1453.147	3	0	1	-217.343	4	0	1	-.215	4	0	1
363		11	max	3838.973	1	0	1	0	1	0	1	0	1	0	1
364			min	-1453.019	3	0	1	-217.491	4	0	1	-.24	4	0	1
365		12	max	3839.143	1	0	1	0	1	0	1	0	1	0	1
366			min	-1452.892	3	0	1	-217.639	4	0	1	-.265	4	0	1
367		13	max	3839.314	1	0	1	0	1	0	1	0	1	0	1
368			min	-1452.764	3	0	1	-217.786	4	0	1	-.29	4	0	1
369		14	max	3839.484	1	0	1	0	1	0	1	0	1	0	1
370			min	-1452.636	3	0	1	-217.934	4	0	1	-.315	4	0	1
371		15	max	3839.654	1	0	1	0	1	0	1	0	1	0	1
372			min	-1452.508	3	0	1	-218.081	4	0	1	-.34	4	0	1
373		16	max	3839.825	1	0	1	0	1	0	1	0	1	0	1
374			min	-1452.38	3	0	1	-218.229	4	0	1	-.365	4	0	1
375		17	max	3839.995	1	0	1	0	1	0	1	0	1	0	1
376			min	-1452.253	3	0	1	-218.377	4	0	1	-.39	4	0	1
377		18	max	3840.165	1	0	1	0	1	0	1	0	1	0	1
378			min	-1452.125	3	0	1	-218.524	4	0	1	-.415	4	0	1
379		19	max	3840.336	1	0	1	0	1	0	1	0	1	0	1
380			min	-1451.997	3	0	1	-218.672	4	0	1	-.44	4	0	1
381	M10	1	max	1167.571	1	2.229	6	-.045	12	0	1	0	1	0	1
382			min	-1367.164	3	.502	15	-71.724	4	0	5	0	3	0	1
383		2	max	1167.9	1	2.214	6	-.045	12	0	1	0	10	0	15
384			min	-1366.918	3	.498	15	-72.009	4	0	5	-.016	4	0	6
385		3	max	1168.228	1	2.199	6	-.045	12	0	1	0	10	0	15
386			min	-1366.671	3	.494	15	-72.293	4	0	5	-.032	4	0	6
387		4	max	1168.556	1	2.183	6	-.045	12	0	1	0	12	0	15
388			min	-1366.425	3	.491	15	-72.578	4	0	5	-.048	4	-.001	6
389		5	max	1168.885	1	2.168	6	-.045	12	0	1	0	12	0	15
390			min	-1366.179	3	.487	15	-72.863	4	0	5	-.064	4	-.002	6
391		6	max	1169.213	1	2.153	6	-.045	12	0	1	0	12	0	15
392			min	-1365.932	3	.484	15	-73.148	4	0	5	-.08	4	-.002	6
393		7	max	1169.542	1	2.138	6	-.045	12	0	1	0	12	0	15
394			min	-1365.686	3	.48	15	-73.433	4	0	5	-.096	4	-.003	6
395		8	max	1169.87	1	2.122	6	-.045	12	0	1	0	12	0	15
396			min	-1365.44	3	.477	15	-73.718	4	0	5	-.113	4	-.003	6
397		9	max	1170.199	1	2.107	6	-.045	12	0	1	0	12	0	15
398			min	-1365.193	3	.473	15	-74.002	4	0	5	-.129	4	-.004	6
399		10	max	1170.527	1	2.092	6	-.045	12	0	1	0	12	0	15
400			min	-1364.947	3	.469	15	-74.287	4	0	5	-.146	4	-.004	6
401		11	max	1170.855	1	2.077	6	-.045	12	0	1	0	12	-.001	15
402			min	-1364.701	3	.466	15	-74.572	4	0	5	-.162	4	-.005	6
403		12	max	1171.184	1	2.061	6	-.045	12	0	1	0	12	-.001	15
404			min	-1364.454	3	.462	15	-74.857	4	0	5	-.179	4	-.005	6
405		13	max	1171.512	1	2.046	6	-.045	12	0	1	0	12	-.001	15
406			min	-1364.208	3	.459	15	-75.142	4	0	5	-.195	4	-.006	6
407		14	max	1171.841	1	2.031	6	-.045	12	0	1	0	12	-.001	15
408			min	-1363.962	3	.455	15	-75.427	4	0	5	-.212	4	-.006	6
409		15	max	1172.169	1	2.016	6	-.045	12	0	1	0	12	-.001	15
410			min	-1363.715	3	.451	15	-75.711	4	0	5	-.229	4	-.007	6
411		16	max	1172.498	1	2	6	-.045	12	0	1	0	12	-.002	15
412			min	-1363.469	3	.448	15	-75.996	4	0	5	-.245	4	-.007	6
413		17	max	1172.826	1	1.985	6	-.045	12	0	1	0	12	-.002	15
414			min	-1363.223	3	.444	15	-76.281	4	0	5	-.262	4	-.007	6
415		18	max	1173.155	1	1.97	6	-.045	12	0	1	0	12	-.002	15
416			min	-1362.976	3	.441	15	-76.566	4	0	5	-.279	4	-.008	6
417		19	max	1173.483	1	1.955	6	-.045	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1362.73	3	.437	15	-76.851	4	0	5	-.296	4	-.008	6
419	M11	1	max	406.878	2	8.051	6	-.001	12	0	1	0	12	.008	6
420			min	-526.338	3	1.881	15	-1.166	5	0	4	-.011	4	.002	15
421		2	max	406.708	2	7.278	6	-.001	12	0	1	0	12	.005	2
422			min	-526.466	3	1.699	15	-.624	5	0	4	-.011	4	.001	12
423		3	max	406.538	2	6.506	6	.006	14	0	1	0	12	.003	2
424			min	-526.593	3	1.518	15	-.082	5	0	4	-.012	4	0	3
425		4	max	406.367	2	5.734	6	.505	4	0	1	0	12	0	2
426			min	-526.721	3	1.336	15	-.017	1	0	4	-.012	4	-.002	3
427		5	max	406.197	2	4.961	6	1.047	4	0	1	0	12	0	15
428			min	-526.849	3	1.154	15	-.017	1	0	4	-.011	4	-.003	3
429		6	max	406.027	2	4.189	6	1.589	4	0	1	0	12	-.001	15
430			min	-526.977	3	.973	15	-.017	1	0	4	-.011	4	-.005	4
431		7	max	405.856	2	3.416	6	2.131	4	0	1	0	12	-.002	15
432			min	-527.104	3	.791	15	-.017	1	0	4	-.01	4	-.006	4
433		8	max	405.686	2	2.644	6	2.673	4	0	1	0	12	-.002	15
434			min	-527.232	3	.61	15	-.017	1	0	4	-.009	4	-.007	4
435		9	max	405.516	2	1.872	6	3.215	4	0	1	0	12	-.002	15
436			min	-527.36	3	.428	15	-.017	1	0	4	-.008	4	-.008	4
437		10	max	405.345	2	1.099	6	3.758	4	0	1	0	12	-.002	15
438			min	-527.488	3	.247	15	-.017	1	0	4	-.006	4	-.009	4
439		11	max	405.175	2	.446	2	4.3	4	0	1	0	12	-.002	15
440			min	-527.615	3	-.026	3	-.017	1	0	4	-.004	4	-.009	4
441		12	max	405.005	2	-.117	15	4.842	4	0	1	0	12	-.002	15
442			min	-527.743	3	-.478	3	-.017	1	0	4	-.003	4	-.009	4
443		13	max	404.834	2	-.298	15	5.384	4	0	1	0	12	-.002	15
444			min	-527.871	3	-1.219	4	-.017	1	0	4	0	5	-.009	4
445		14	max	404.664	2	-.48	15	5.926	4	0	1	.002	4	-.002	15
446			min	-527.999	3	-1.992	4	-.017	1	0	4	0	1	-.008	4
447		15	max	404.494	2	-.661	15	6.468	4	0	1	.005	4	-.002	15
448			min	-528.126	3	-2.764	4	-.017	1	0	4	0	1	-.007	4
449		16	max	404.323	2	-.843	15	7.01	4	0	1	.007	4	-.001	15
450			min	-528.254	3	-3.537	4	-.017	1	0	4	0	1	-.006	4
451		17	max	404.153	2	-1.024	15	7.552	4	0	1	.011	4	-.001	15
452			min	-528.382	3	-4.309	4	-.017	1	0	4	0	1	-.004	4
453		18	max	403.983	2	-1.206	15	8.095	4	0	1	.014	4	0	15
454			min	-528.51	3	-5.081	4	-.017	1	0	4	0	1	-.002	4
455		19	max	403.812	2	-1.388	15	8.637	4	0	1	.017	4	0	1
456			min	-528.637	3	-5.854	4	-.017	1	0	4	0	1	0	1
457	M12	1	max	1296.071	1	0	1	6.788	1	0	1	.009	4	0	1
458			min	-433.167	3	0	1	-217.362	4	0	1	0	1	0	1
459		2	max	1296.241	1	0	1	6.788	1	0	1	0	1	0	1
460			min	-433.039	3	0	1	-217.51	4	0	1	-.016	4	0	1
461		3	max	1296.412	1	0	1	6.788	1	0	1	.001	1	0	1
462			min	-432.912	3	0	1	-217.657	4	0	1	-.041	4	0	1
463		4	max	1296.582	1	0	1	6.788	1	0	1	.002	1	0	1
464			min	-432.784	3	0	1	-217.805	4	0	1	-.066	4	0	1
465		5	max	1296.752	1	0	1	6.788	1	0	1	.003	1	0	1
466			min	-432.656	3	0	1	-217.952	4	0	1	-.091	4	0	1
467		6	max	1296.923	1	0	1	6.788	1	0	1	.004	1	0	1
468			min	-432.528	3	0	1	-218.1	4	0	1	-.116	4	0	1
469		7	max	1297.093	1	0	1	6.788	1	0	1	.005	1	0	1
470			min	-432.401	3	0	1	-218.248	4	0	1	-.141	4	0	1
471		8	max	1297.263	1	0	1	6.788	1	0	1	.005	1	0	1
472			min	-432.273	3	0	1	-218.395	4	0	1	-.166	4	0	1
473		9	max	1297.434	1	0	1	6.788	1	0	1	.006	1	0	1
474			min	-432.145	3	0	1	-218.543	4	0	1	-.191	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1297.604	1	0	1	6.788	1	0	1	.007	1	0	1
476			min	-432.017	3	0	1	-218.691	4	0	1	-.216	4	0	1
477		11	max	1297.775	1	0	1	6.788	1	0	1	.008	1	0	1
478			min	-431.89	3	0	1	-218.838	4	0	1	-.241	4	0	1
479		12	max	1297.945	1	0	1	6.788	1	0	1	.008	1	0	1
480			min	-431.762	3	0	1	-218.986	4	0	1	-.266	4	0	1
481		13	max	1298.115	1	0	1	6.788	1	0	1	.009	1	0	1
482			min	-431.634	3	0	1	-219.134	4	0	1	-.292	4	0	1
483		14	max	1298.286	1	0	1	6.788	1	0	1	.01	1	0	1
484			min	-431.506	3	0	1	-219.281	4	0	1	-.317	4	0	1
485		15	max	1298.456	1	0	1	6.788	1	0	1	.011	1	0	1
486			min	-431.379	3	0	1	-219.429	4	0	1	-.342	4	0	1
487		16	max	1298.626	1	0	1	6.788	1	0	1	.012	1	0	1
488			min	-431.251	3	0	1	-219.576	4	0	1	-.367	4	0	1
489		17	max	1298.797	1	0	1	6.788	1	0	1	.012	1	0	1
490			min	-431.123	3	0	1	-219.724	4	0	1	-.392	4	0	1
491		18	max	1298.967	1	0	1	6.788	1	0	1	.013	1	0	1
492			min	-430.995	3	0	1	-219.872	4	0	1	-.418	4	0	1
493		19	max	1299.137	1	0	1	6.788	1	0	1	.014	1	0	1
494			min	-430.868	3	0	1	-220.019	4	0	1	-.443	4	0	1
495	M1	1	max	125.586	1	767.773	3	51.948	5	0	1	.168	1	0	3
496			min	-6.912	5	-529.605	1	-70.883	1	0	3	-.055	5	-.016	2
497		2	max	125.957	1	766.736	3	53.189	5	0	1	.131	1	.265	1
498			min	-6.739	5	-530.989	1	-70.883	1	0	3	-.027	5	-.404	3
499		3	max	314.212	3	603.235	1	-3.375	12	0	3	.094	1	.532	1
500			min	-190.909	2	-568.848	3	-70.043	1	0	1	0	15	-.792	3
501		4	max	314.49	3	601.851	1	-3.375	12	0	3	.057	1	.214	1
502			min	-190.538	2	-569.885	3	-70.043	1	0	1	-.008	5	-.492	3
503		5	max	314.768	3	600.467	1	-3.375	12	0	3	.02	1	-.004	15
504			min	-190.168	2	-570.923	3	-70.043	1	0	1	-.015	5	-.191	3
505		6	max	315.046	3	599.084	1	-3.375	12	0	3	0	12	.111	3
506			min	-189.797	2	-571.961	3	-70.043	1	0	1	-.025	4	-.427	2
507		7	max	315.324	3	597.7	1	-3.375	12	0	3	-.003	12	.413	3
508			min	-189.426	2	-572.998	3	-70.043	1	0	1	-.054	1	-.736	1
509		8	max	315.602	3	596.317	1	-3.375	12	0	3	-.004	12	.715	3
510			min	-189.055	2	-574.036	3	-70.043	1	0	1	-.091	1	-1.051	1
511		9	max	323.227	3	51.317	2	36.868	5	0	9	.055	1	.834	3
512			min	-139.073	2	.417	15	-104.764	1	0	3	-.108	5	-1.198	1
513		10	max	323.505	3	49.933	2	38.109	5	0	9	0	10	.814	3
514			min	-138.702	2	0	5	-104.764	1	0	3	-.088	4	-1.212	1
515		11	max	323.783	3	48.549	2	39.351	5	0	9	-.003	12	.794	3
516			min	-138.331	2	-1.74	4	-104.764	1	0	3	-.079	4	-1.226	2
517		12	max	331.332	3	384.146	3	117.851	5	0	2	.09	1	.693	3
518			min	-88.32	2	-671.786	2	-68.532	1	0	3	-.16	5	-1.087	2
519		13	max	331.61	3	383.108	3	119.092	5	0	2	.054	1	.49	3
520			min	-87.949	2	-673.17	2	-68.532	1	0	3	-.097	5	-.739	1
521		14	max	331.888	3	382.071	3	120.334	5	0	2	.018	1	.288	3
522			min	-87.578	2	-674.554	2	-68.532	1	0	3	-.034	5	-.394	1
523		15	max	332.166	3	381.033	3	121.575	5	0	2	.03	5	.087	3
524			min	-87.207	2	-675.937	2	-68.532	1	0	3	-.018	1	-.049	1
525		16	max	332.444	3	379.995	3	122.817	5	0	2	.094	5	.337	2
526			min	-86.837	2	-677.321	2	-68.532	1	0	3	-.054	1	-.114	3
527		17	max	332.722	3	378.958	3	124.058	5	0	2	.159	5	.695	2
528			min	-86.466	2	-678.704	2	-68.532	1	0	3	-.091	1	-.314	3
529		18	max	14.565	5	684.694	2	-3.349	12	0	5	.136	5	.349	2
530			min	-126.162	1	-316.817	3	-97.853	4	0	2	-.13	1	-.155	3
531		19	max	14.738	5	683.31	2	-3.349	12	0	5	.093	5	.012	3





Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532	M5	min	-125.791	1	-317.854	3	-96.611	4	0	2	-.169	1	-.013	1
533		max	273.579	1	2560.505	3	78.828	5	0	1	0	1	.032	2
534		min	7.704	12	-1801.315	1	0	1	0	4	-.12	4	-.001	3
535		max	273.95	1	2559.468	3	80.069	5	0	1	0	1	.981	1
536		min	7.89	12	-1802.699	1	0	1	0	4	-.079	4	-1.352	3
537		max	1005.123	3	1812.646	1	13.865	4	0	4	0	1	1.889	1
538		min	-661.981	2	-1798.261	3	0	1	0	1	-.037	4	-2.65	3
539		max	1005.401	3	1811.262	1	15.107	4	0	4	0	1	.933	1
540		min	-661.61	2	-1799.299	3	0	1	0	1	-.03	4	-1.701	3
541		max	1005.679	3	1809.879	1	16.348	4	0	4	0	1	.019	9
542	M6	min	-661.239	2	-1800.337	3	0	1	0	1	-.021	4	-.751	3
543		max	1005.957	3	1808.495	1	17.59	4	0	4	0	1	.199	3
544		min	-660.868	2	-1801.374	3	0	1	0	1	-.013	5	-.998	2
545		max	1006.235	3	1807.111	1	18.831	4	0	4	0	1	1.15	3
546		min	-660.498	2	-1802.412	3	0	1	0	1	-.004	5	-1.931	1
547		max	1006.513	3	1805.728	1	20.072	4	0	4	.007	4	2.101	3
548		min	-660.127	2	-1803.45	3	0	1	0	1	0	1	-2.884	1
549		max	1017.757	3	172.013	2	118.063	4	0	1	0	1	2.416	3
550		min	-556.006	2	.417	15	0	1	0	1	-.142	4	-3.267	1
551		max	1018.035	3	170.63	2	119.304	4	0	1	0	1	2.343	3
552	M7	min	-555.636	2	0	15	0	1	0	1	-.079	5	-3.314	1
553		max	1018.313	3	169.246	2	120.546	4	0	1	0	1	2.271	3
554		min	-555.265	2	-1.628	6	0	1	0	1	-.017	5	-3.367	2
555		max	1029.708	3	1193.046	3	160.184	4	0	1	0	1	1.994	3
556		min	-451.202	2	-2052.465	2	0	1	0	4	-.221	4	-3.015	2
557		max	1029.986	3	1192.009	3	161.425	4	0	1	0	1	1.364	3
558		min	-450.831	2	-2053.849	2	0	1	0	4	-.136	4	-1.949	1
559		max	1030.264	3	1190.971	3	162.667	4	0	1	0	1	.736	3
560		min	-450.46	2	-2055.232	2	0	1	0	4	-.051	4	-.9	1
561		max	1030.542	3	1189.933	3	163.908	4	0	1	.035	4	.238	2
562	M8	min	-450.089	2	-2056.616	2	0	1	0	4	0	1	-.003	13
563		max	1030.82	3	1188.895	3	165.15	4	0	1	.122	4	1.323	2
564		min	-449.719	2	-2058	2	0	1	0	4	0	1	-.52	3
565		max	1031.098	3	1187.858	3	166.391	4	0	1	.21	4	2.41	2
566		min	-449.348	2	-2059.383	2	0	1	0	4	0	1	-1.147	3
567		max	-8.2	12	2311.425	2	0	1	0	4	.21	4	1.242	2
568		min	-273.54	1	-1090.372	3	-37.215	5	0	1	0	1	-.6	3
569		max	-8.014	12	2310.042	2	0	1	0	4	.191	4	.026	1
570		min	-273.169	1	-1091.41	3	-35.974	5	0	1	0	1	-.024	3
571		max	125.586	1	767.773	3	73.984	4	0	3	-.008	12	0	3
572	M9	min	4.701	12	-529.605	1	3.423	12	0	4	-.168	1	-.016	2
573		max	125.957	1	766.736	3	75.226	4	0	3	-.006	12	.265	1
574		min	4.886	12	-530.989	1	3.423	12	0	4	-.131	1	-.404	3
575		max	314.212	3	603.235	1	70.043	1	0	1	-.005	12	.532	1
576		min	-190.909	2	-568.848	3	-8.287	5	0	3	-.094	1	-.792	3
577		max	314.49	3	601.851	1	70.043	1	0	1	-.003	12	.214	1
578		min	-190.538	2	-569.885	3	-7.045	5	0	3	-.057	1	-.492	3
579		max	314.768	3	600.467	1	70.043	1	0	1	0	12	-.004	15
580		min	-190.168	2	-570.923	3	-5.804	5	0	3	-.021	4	-.191	3
581		max	315.046	3	599.084	1	70.043	1	0	1	.017	1	.111	3
582	M10	min	-189.797	2	-571.961	3	-4.562	5	0	3	-.02	5	-.427	2
583		max	315.324	3	597.7	1	70.043	1	0	1	.054	1	.413	3
584		min	-189.426	2	-572.998	3	-3.321	5	0	3	-.022	5	-.736	1
585		max	315.602	3	596.317	1	70.043	1	0	1	.091	1	.715	3
586		min	-189.055	2	-574.036	3	-2.08	5	0	3	-.023	5	-1.051	1
587		max	323.227	3	51.317	2	104.764	1	0	3	-.002	12	.834	3
588		min	-139.073	2	.422	15	4.855	12	0	9	-.125	4	-1.198	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	323.505	3	49.933	2	104.764	1	0	3	0	1	.814	3
590		min	-138.702	2	.005	15	4.855	12	0	9	-.088	4	-1.212	1
591	11	max	323.783	3	48.549	2	104.764	1	0	3	.056	1	.794	3
592		min	-138.331	2	-1.701	6	4.855	12	0	9	-.062	5	-1.226	2
593	12	max	331.332	3	384.146	3	140.056	4	0	3	-.004	12	.693	3
594		min	-88.32	2	-671.786	2	3.049	12	0	2	-.189	4	-1.087	2
595	13	max	331.61	3	383.108	3	141.297	4	0	3	-.002	12	.49	3
596		min	-87.949	2	-673.17	2	3.049	12	0	2	-.114	4	-.739	1
597	14	max	331.888	3	382.071	3	142.539	4	0	3	0	12	.288	3
598		min	-87.578	2	-674.554	2	3.049	12	0	2	-.04	4	-.394	1
599	15	max	332.166	3	381.033	3	143.78	4	0	3	.036	4	.087	3
600		min	-87.207	2	-675.937	2	3.049	12	0	2	0	12	-.049	1
601	16	max	332.444	3	379.995	3	145.022	4	0	3	.112	4	.337	2
602		min	-86.837	2	-677.321	2	3.049	12	0	2	.002	12	-.114	3
603	17	max	332.722	3	378.958	3	146.263	4	0	3	.189	4	.695	2
604		min	-86.466	2	-678.704	2	3.049	12	0	2	.004	12	-.314	3
605	18	max	-4.731	12	684.694	2	75.063	1	0	2	.177	4	.349	2
606		min	-126.162	1	-316.817	3	-74.512	5	0	3	.006	12	-.155	3
607	19	max	-4.546	12	683.31	2	75.063	1	0	2	.169	1	.012	3
608		min	-125.791	1	-317.854	3	-73.27	5	0	3	.007	12	-.013	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.127	2	.006	3	1.021e-2	2	NC	1	NC	1
2			min	-.428	4	-.029	3	-.003	2	-2.243e-3	3	NC	1	NC	1
3		2	max	0	1	.257	3	.024	1	1.163e-2	2	NC	5	NC	2
4			min	-.428	4	-.051	1	-.01	5	-2.305e-3	3	776.078	3	9736.598	1
5		3	max	0	1	.488	3	.056	1	1.306e-2	2	NC	5	NC	2
6			min	-.428	4	-.189	1	-.013	5	-2.366e-3	3	429.071	3	4024.268	1
7		4	max	0	1	.628	3	.084	1	1.449e-2	2	NC	5	NC	3
8			min	-.428	4	-.265	1	-.009	5	-2.428e-3	3	337.75	3	2677.131	1
9		5	max	0	1	.659	3	.098	1	1.591e-2	2	NC	5	NC	3
10			min	-.428	4	-.266	1	-.002	5	-2.49e-3	3	322.394	3	2289.712	1
11		6	max	0	1	.584	3	.094	1	1.734e-2	2	NC	5	NC	3
12			min	-.428	4	-.195	1	.003	15	-2.552e-3	3	361.685	3	2382.448	1
13		7	max	0	1	.426	3	.074	1	1.877e-2	2	NC	5	NC	3
14			min	-.428	4	-.068	1	.001	10	-2.614e-3	3	487.965	3	3058.993	1
15		8	max	0	1	.224	3	.042	1	2.019e-2	2	NC	2	NC	2
16			min	-.428	4	.002	15	-.002	10	-2.676e-3	3	874.87	3	5395.329	1
17		9	max	0	1	.235	2	.018	3	2.162e-2	2	NC	4	NC	1
18			min	-.428	4	.005	15	-.006	10	-2.737e-3	3	2049.063	2	NC	1
19		10	max	0	1	.29	2	.017	3	2.305e-2	2	NC	3	NC	1
20			min	-.428	4	-.04	3	-.011	2	-2.799e-3	3	1361.355	2	NC	1
21		11	max	0	12	.235	2	.018	3	2.162e-2	2	NC	4	NC	1
22			min	-.428	4	.005	15	-.008	5	-2.737e-3	3	2049.063	2	NC	1
23		12	max	0	12	.224	3	.042	1	2.019e-2	2	NC	2	NC	2
24			min	-.428	4	.002	15	-.008	5	-2.676e-3	3	874.87	3	5395.329	1
25		13	max	0	12	.426	3	.074	1	1.877e-2	2	NC	5	NC	3
26			min	-.428	4	-.068	1	-.003	5	-2.614e-3	3	487.965	3	3058.993	1
27		14	max	0	12	.584	3	.094	1	1.734e-2	2	NC	5	NC	3
28			min	-.428	4	-.195	1	.003	15	-2.552e-3	3	361.685	3	2382.448	1
29		15	max	0	12	.659	3	.098	1	1.591e-2	2	NC	5	NC	3
30			min	-.428	4	-.266	1	.005	10	-2.49e-3	3	322.394	3	2289.712	1
31		16	max	0	12	.628	3	.084	1	1.449e-2	2	NC	5	NC	3
32			min	-.428	4	-.265	1	.004	10	-2.428e-3	3	337.75	3	2677.131	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.488	3	.056	1	1.306e-2	2	NC	5	NC	2
34		min	-.428	4	-.189	1	.002	10	-2.366e-3	3	429.071	3	4024.268	1
35	18	max	0	12	.257	3	.024	1	1.163e-2	2	NC	5	NC	2
36		min	-.428	4	-.051	1	0	10	-2.305e-3	3	776.078	3	9736.598	1
37	19	max	0	12	.127	2	.006	3	1.021e-2	2	NC	1	NC	1
38		min	-.428	4	-.029	3	-.003	2	-2.243e-3	3	NC	1	NC	1
39	M14	1	max	0	.248	3	.005	3	6.057e-3	1	NC	1	NC	1
40		min	-.345	4	-.394	1	-.002	2	-4.442e-3	3	NC	1	NC	1
41	2	max	0	1	.544	3	.016	1	7.223e-3	1	NC	5	NC	1
42		min	-.345	4	-.704	1	-.015	5	-5.37e-3	3	715.773	1	NC	1
43	3	max	0	1	.795	3	.045	1	8.389e-3	1	NC	5	NC	2
44		min	-.345	4	-.973	1	-.018	5	-6.297e-3	3	383.248	1	5080.22	1
45	4	max	0	1	.973	3	.071	1	9.555e-3	1	NC	15	NC	3
46		min	-.345	4	-1.173	1	-.013	5	-7.225e-3	3	285.032	1	3154.941	1
47	5	max	0	1	1.062	3	.087	1	1.072e-2	1	NC	15	NC	3
48		min	-.345	4	-1.288	1	-.002	5	-8.152e-3	3	248.259	1	2598.259	1
49	6	max	0	1	1.061	3	.085	1	1.189e-2	1	NC	15	NC	3
50		min	-.345	4	-1.319	1	.003	10	-9.08e-3	3	240.161	1	2638.669	1
51	7	max	0	1	.987	3	.068	1	1.305e-2	1	NC	15	NC	2
52		min	-.345	4	-1.276	1	.001	10	-1.001e-2	3	251.682	1	3328.67	1
53	8	max	0	1	.869	3	.04	1	1.422e-2	1	NC	15	NC	2
54		min	-.345	4	-1.188	1	-.002	10	-1.094e-2	3	279.682	1	5778.022	1
55	9	max	0	1	.752	3	.021	4	1.538e-2	1	NC	15	NC	1
56		min	-.345	4	-1.094	1	-.005	10	-1.186e-2	3	317.287	1	NC	1
57	10	max	0	1	.696	3	.016	3	1.655e-2	1	NC	5	NC	1
58		min	-.345	4	-1.048	1	-.01	2	-1.279e-2	3	339.559	1	NC	1
59	11	max	0	12	.752	3	.016	3	1.538e-2	1	NC	15	NC	1
60		min	-.345	4	-1.094	1	-.015	5	-1.186e-2	3	317.287	1	NC	1
61	12	max	0	12	.869	3	.04	1	1.422e-2	1	NC	15	NC	2
62		min	-.345	4	-1.188	1	-.018	5	-1.094e-2	3	279.682	1	5778.022	1
63	13	max	0	12	.987	3	.068	1	1.305e-2	1	NC	15	NC	2
64		min	-.345	4	-1.276	1	-.011	5	-1.001e-2	3	251.682	1	3328.67	1
65	14	max	0	12	1.061	3	.085	1	1.189e-2	1	NC	15	NC	3
66		min	-.345	4	-1.319	1	0	5	-9.08e-3	3	240.161	1	2638.669	1
67	15	max	0	12	1.062	3	.087	1	1.072e-2	1	NC	15	NC	3
68		min	-.345	4	-1.288	1	.004	10	-8.152e-3	3	248.259	1	2598.259	1
69	16	max	0	12	.973	3	.071	1	9.555e-3	1	NC	15	NC	3
70		min	-.345	4	-1.173	1	.003	10	-7.225e-3	3	285.032	1	3154.941	1
71	17	max	0	12	.795	3	.045	1	8.389e-3	1	NC	5	NC	2
72		min	-.345	4	-.973	1	.002	10	-6.297e-3	3	383.248	1	5080.22	1
73	18	max	0	12	.544	3	.021	4	7.223e-3	1	NC	5	NC	1
74		min	-.345	4	-.704	1	0	10	-5.37e-3	3	715.773	1	NC	1
75	19	max	0	12	.248	3	.005	3	6.057e-3	1	NC	1	NC	1
76		min	-.345	4	-.394	1	-.002	2	-4.442e-3	3	NC	1	NC	1
77	M15	1	max	0	.254	3	.005	3	3.778e-3	3	NC	1	NC	1
78		min	-.293	4	-.394	1	-.002	2	-6.179e-3	1	NC	1	NC	1
79	2	max	0	12	.451	3	.016	1	4.567e-3	3	NC	5	NC	1
80		min	-.293	4	-.755	2	-.023	5	-7.373e-3	1	607.786	2	9124.15	5
81	3	max	0	12	.623	3	.045	1	5.356e-3	3	NC	5	NC	2
82		min	-.293	4	-1.069	2	-.029	5	-8.567e-3	1	327.067	2	5063.914	1
83	4	max	0	12	.753	3	.072	1	6.145e-3	3	NC	15	NC	3
84		min	-.293	4	-1.295	2	-.021	5	-9.761e-3	1	245.343	2	3146.343	1
85	5	max	0	12	.832	3	.087	1	6.934e-3	3	NC	15	NC	3
86		min	-.293	4	-1.416	2	-.006	5	-1.095e-2	1	216.483	2	2591.197	1
87	6	max	0	12	.858	3	.085	1	7.723e-3	3	NC	15	NC	3
88		min	-.293	4	-1.43	2	.004	10	-1.215e-2	1	213.425	2	2630.429	1
89	7	max	0	12	.839	3	.068	1	8.512e-3	3	NC	15	NC	2





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.293	4	-1.356	2	.001	10	-1.334e-2	1	229.806	2	3314.482	1
91	8	max	0	12	.791	3	.04	4	9.301e-3	3	NC	15	NC	2
92		min	-.293	4	-1.228	2	-.002	10	-1.454e-2	1	264.999	2	5490.237	4
93	9	max	0	12	.738	3	.028	4	1.009e-2	3	NC	15	NC	1
94		min	-.293	4	-1.104	1	-.005	10	-1.573e-2	1	312.623	1	7966.672	4
95	10	max	0	1	.711	3	.015	3	1.088e-2	3	NC	5	NC	1
96		min	-.293	4	-1.047	1	-.009	2	-1.692e-2	1	339.845	1	NC	1
97	11	max	0	1	.738	3	.015	3	1.009e-2	3	NC	15	NC	1
98		min	-.293	4	-1.104	1	-.022	5	-1.573e-2	1	312.623	1	9890.99	5
99	12	max	0	1	.791	3	.04	1	9.301e-3	3	NC	15	NC	2
100		min	-.293	4	-1.228	2	-.026	5	-1.454e-2	1	264.999	2	5733.293	1
101	13	max	0	1	.839	3	.068	1	8.512e-3	3	NC	15	NC	2
102		min	-.293	4	-1.356	2	-.018	5	-1.334e-2	1	229.806	2	3314.482	1
103	14	max	0	1	.858	3	.085	1	7.723e-3	3	NC	15	NC	3
104		min	-.293	4	-1.43	2	-.002	5	-1.215e-2	1	213.425	2	2630.429	1
105	15	max	0	1	.832	3	.087	1	6.934e-3	3	NC	15	NC	3
106		min	-.293	4	-1.416	2	.004	10	-1.095e-2	1	216.483	2	2591.197	1
107	16	max	0	1	.753	3	.072	1	6.145e-3	3	NC	15	NC	3
108		min	-.293	4	-1.295	2	.004	10	-9.761e-3	1	245.343	2	3146.343	1
109	17	max	0	1	.623	3	.045	1	5.356e-3	3	NC	5	NC	2
110		min	-.293	4	-1.069	2	.002	10	-8.567e-3	1	327.067	2	5063.914	1
111	18	max	0	1	.451	3	.029	4	4.567e-3	3	NC	5	NC	1
112		min	-.293	4	-.755	2	0	10	-7.373e-3	1	607.786	2	7527.385	4
113	19	max	0	1	.254	3	.005	3	3.778e-3	3	NC	1	NC	1
114		min	-.293	4	-.394	1	-.002	2	-6.179e-3	1	NC	1	NC	1
115	M16	1	max	0	.118	1	.004	3	6.688e-3	3	NC	1	NC	1
116		min	-.133	4	-.085	3	-.002	2	-9.079e-3	1	NC	1	NC	1
117	2	max	0	12	.015	3	.024	1	7.77e-3	3	NC	5	NC	2
118		min	-.133	4	-.126	2	-.017	5	-1.024e-2	1	937.73	2	9781.414	1
119	3	max	0	12	.093	3	.056	1	8.851e-3	3	NC	5	NC	2
120		min	-.133	4	-.314	2	-.022	5	-1.14e-2	1	522.466	2	4028.647	1
121	4	max	0	12	.133	3	.084	1	9.933e-3	3	NC	5	NC	3
122		min	-.133	4	-.421	2	-.017	5	-1.256e-2	1	417.263	2	2674.002	1
123	5	max	0	12	.129	3	.098	1	1.101e-2	3	NC	5	NC	3
124		min	-.133	4	-.432	2	-.007	5	-1.372e-2	1	408.647	2	2281.944	1
125	6	max	0	12	.082	3	.095	1	1.21e-2	3	NC	5	NC	3
126		min	-.133	4	-.35	2	.003	15	-1.488e-2	1	481.281	2	2367.142	1
127	7	max	0	12	.003	12	.074	1	1.318e-2	3	NC	5	NC	3
128		min	-.133	4	-.196	2	.002	10	-1.604e-2	1	724.622	2	3022.422	1
129	8	max	0	12	.031	1	.043	1	1.426e-2	3	NC	3	NC	2
130		min	-.133	4	-.092	3	0	10	-1.72e-2	1	1906.866	2	5252.324	1
131	9	max	0	12	.19	1	.019	4	1.534e-2	3	NC	4	NC	1
132		min	-.133	4	-.175	3	-.004	10	-1.837e-2	1	2449.959	3	NC	1
133	10	max	0	1	.261	1	.013	3	1.642e-2	3	NC	5	NC	1
134		min	-.133	4	-.212	3	-.009	2	-1.953e-2	1	1552.304	1	NC	1
135	11	max	0	1	.19	1	.013	3	1.534e-2	3	NC	4	NC	1
136		min	-.133	4	-.175	3	-.014	5	-1.837e-2	1	2449.959	3	NC	1
137	12	max	0	1	.031	1	.043	1	1.426e-2	3	NC	3	NC	2
138		min	-.133	4	-.092	3	-.015	5	-1.72e-2	1	1906.866	2	5252.324	1
139	13	max	0	1	.003	12	.074	1	1.318e-2	3	NC	5	NC	3
140		min	-.133	4	-.196	2	-.007	5	-1.604e-2	1	724.622	2	3022.422	1
141	14	max	0	1	.082	3	.095	1	1.21e-2	3	NC	5	NC	3
142		min	-.133	4	-.35	2	.003	15	-1.488e-2	1	481.281	2	2367.142	1
143	15	max	0	1	.129	3	.098	1	1.101e-2	3	NC	5	NC	3
144		min	-.133	4	-.432	2	.006	10	-1.372e-2	1	408.647	2	2281.944	1
145	16	max	0	1	.133	3	.084	1	9.933e-3	3	NC	5	NC	3
146		min	-.133	4	-.421	2	.005	10	-1.256e-2	1	417.263	2	2674.002	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.093	3	.056	1	8.851e-3	3	NC	5	NC	2
148			min	-1.133	4	-.314	2	.003	10	-1.14e-2	1	522.466	2	4028.647	1
149		18	max	0	1	.015	3	.025	4	7.77e-3	3	NC	5	NC	2
150			min	-1.133	4	-.126	2	0	10	-1.024e-2	1	937.73	2	8677.498	4
151		19	max	0	1	.118	1	.004	3	6.688e-3	3	NC	1	NC	1
152			min	-1.133	4	-.085	3	-.002	2	-9.079e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.004	2	.005	1	1.148e-3	5	NC	1	NC	2
154			min	-.006	3	-.007	3	-.406	4	-1.376e-4	1	NC	1	117.787	4
155		2	max	.005	1	.003	2	.005	1	1.222e-3	5	NC	1	NC	2
156			min	-.006	3	-.007	3	-.373	4	-1.275e-4	1	NC	1	128.34	4
157		3	max	.005	1	.003	2	.005	1	1.296e-3	5	NC	1	NC	1
158			min	-.006	3	-.007	3	-.34	4	-1.174e-4	1	NC	1	140.887	4
159		4	max	.004	1	.002	2	.004	1	1.37e-3	5	NC	1	NC	1
160			min	-.005	3	-.006	3	-.307	4	-1.073e-4	1	NC	1	155.953	4
161		5	max	.004	1	.002	2	.004	1	1.444e-3	5	NC	1	NC	1
162			min	-.005	3	-.006	3	-.275	4	-9.724e-5	1	NC	1	174.246	4
163		6	max	.004	1	.001	2	.003	1	1.518e-3	5	NC	1	NC	1
164			min	-.005	3	-.006	3	-.243	4	-8.715e-5	1	NC	1	196.755	4
165		7	max	.004	1	0	2	.003	1	1.592e-3	5	NC	1	NC	1
166			min	-.004	3	-.005	3	-.213	4	-7.706e-5	1	NC	1	224.881	4
167		8	max	.003	1	0	2	.002	1	1.667e-3	4	NC	1	NC	1
168			min	-.004	3	-.005	3	-.184	4	-6.698e-5	1	NC	1	260.674	4
169		9	max	.003	1	0	2	.002	1	1.745e-3	4	NC	1	NC	1
170			min	-.003	3	-.005	3	-.156	4	-5.689e-5	1	NC	1	307.224	4
171		10	max	.003	1	0	2	.002	1	1.823e-3	4	NC	1	NC	1
172			min	-.003	3	-.004	3	-.13	4	-4.68e-5	1	NC	1	369.374	4
173		11	max	.002	1	0	15	.001	1	1.9e-3	4	NC	1	NC	1
174			min	-.003	3	-.004	3	-.105	4	-3.672e-5	1	NC	1	455.069	4
175		12	max	.002	1	0	15	.001	1	1.978e-3	4	NC	1	NC	1
176			min	-.002	3	-.004	3	-.083	4	-2.663e-5	1	NC	1	578.1	4
177		13	max	.002	1	0	15	0	1	2.056e-3	4	NC	1	NC	1
178			min	-.002	3	-.003	3	-.063	4	-1.654e-5	1	NC	1	764.182	4
179		14	max	.001	1	0	15	0	1	2.133e-3	4	NC	1	NC	1
180			min	-.002	3	-.003	3	-.045	4	-6.458e-6	1	NC	1	1065.927	4
181		15	max	.001	1	0	15	0	1	2.211e-3	4	NC	1	NC	1
182			min	-.001	3	-.002	3	-.03	4	-3.171e-7	3	NC	1	1605.59	4
183		16	max	0	1	0	15	0	1	2.289e-3	4	NC	1	NC	1
184			min	-.001	3	-.002	3	-.018	4	3.126e-7	12	NC	1	2725.05	4
185		17	max	0	1	0	15	0	1	2.367e-3	4	NC	1	NC	1
186			min	0	3	-.001	3	-.008	4	8.046e-7	12	NC	1	5716.003	4
187		18	max	0	1	0	15	0	1	2.444e-3	4	NC	1	NC	1
188			min	0	3	0	3	-.002	4	1.297e-6	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.522e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.789e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-5.681e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-5.631e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.012	4	6.631e-6	4	NC	1	NC	1
194			min	0	2	-.001	6	0	12	1.759e-7	12	NC	1	NC	1
195		3	max	0	3	0	15	.024	4	5.764e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	9.2e-7	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.035	4	1.146e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	1.664e-6	12	NC	1	NC	1
199		5	max	.001	3	-.001	15	.046	4	1.716e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	2.408e-6	12	NC	1	NC	1
201		6	max	.001	3	-.002	15	.056	4	2.286e-3	4	NC	1	NC	1
202			min	0	2	-.008	6	0	12	3.152e-6	12	NC	1	9775.437	5
203		7	max	.002	3	-.002	15	.066	4	2.855e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.001	2	-.01	6	0	12	3.896e-6	12	9471.44	6	9403.952	5
205		8	max	.002	3	-.002	15	.075	4	3.425e-3	4	NC	1	NC	1
206			min	-.001	2	-.011	6	0	12	4.64e-6	12	8429.61	6	9475.827	5
207		9	max	.002	3	-.003	15	.084	4	3.995e-3	4	NC	1	NC	1
208			min	-.002	2	-.012	6	0	12	5.384e-6	12	7805.877	6	9962.811	5
209		10	max	.002	3	-.003	15	.092	4	4.565e-3	4	NC	2	NC	1
210			min	-.002	2	-.012	6	0	12	6.128e-6	12	7488.844	6	NC	1
211		11	max	.003	3	-.003	15	.101	4	5.135e-3	4	NC	2	NC	1
212			min	-.002	2	-.012	6	0	12	6.872e-6	12	7430.757	6	NC	1
213		12	max	.003	3	-.003	15	.109	4	5.704e-3	4	NC	1	NC	1
214			min	-.002	2	-.012	6	0	12	7.616e-6	12	7628.999	6	NC	1
215		13	max	.003	3	-.002	15	.116	4	6.274e-3	4	NC	1	NC	1
216			min	-.002	2	-.011	6	0	12	8.36e-6	12	8127.308	6	NC	1
217		14	max	.003	3	-.002	15	.124	4	6.844e-3	4	NC	1	NC	1
218			min	-.003	2	-.01	6	0	12	9.104e-6	12	9039.245	6	NC	1
219		15	max	.004	3	-.002	15	.132	4	7.414e-3	4	NC	1	NC	1
220			min	-.003	2	-.009	6	0	12	9.848e-6	12	NC	1	NC	1
221		16	max	.004	3	-.001	15	.141	4	7.983e-3	4	NC	1	NC	1
222			min	-.003	2	-.008	1	0	12	1.059e-5	12	NC	1	NC	1
223		17	max	.004	3	0	15	.149	4	8.553e-3	4	NC	1	NC	1
224			min	-.003	2	-.006	1	0	12	1.134e-5	12	NC	1	NC	1
225		18	max	.004	3	0	15	.158	4	9.123e-3	4	NC	1	NC	1
226			min	-.003	2	-.005	1	0	12	1.208e-5	12	NC	1	NC	1
227		19	max	.005	3	0	5	.168	4	9.693e-3	4	NC	1	NC	1
228			min	-.004	2	-.003	1	0	12	1.282e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.003	2	0	12	-1.432e-7	12	NC	1	NC	2
230			min	-.001	3	-.005	3	-.168	4	-7.218e-4	4	NC	1	147.601	4
231		2	max	.003	1	.003	2	0	12	-1.432e-7	12	NC	1	NC	2
232			min	0	3	-.004	3	-.154	4	-7.218e-4	4	NC	1	160.681	4
233		3	max	.003	1	.003	2	0	12	-1.432e-7	12	NC	1	NC	2
234			min	0	3	-.004	3	-.141	4	-7.218e-4	4	NC	1	176.237	4
235		4	max	.003	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	2
236			min	0	3	-.004	3	-.127	4	-7.218e-4	4	NC	1	194.913	4
237		5	max	.002	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	2
238			min	0	3	-.004	3	-.114	4	-7.218e-4	4	NC	1	217.587	4
239		6	max	.002	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	2
240			min	0	3	-.003	3	-.101	4	-7.218e-4	4	NC	1	245.473	4
241		7	max	.002	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	2
242			min	0	3	-.003	3	-.088	4	-7.218e-4	4	NC	1	280.297	4
243		8	max	.002	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	1
244			min	0	3	-.003	3	-.076	4	-7.218e-4	4	NC	1	324.576	4
245		9	max	.002	1	.002	2	0	12	-1.432e-7	12	NC	1	NC	1
246			min	0	3	-.003	3	-.065	4	-7.218e-4	4	NC	1	382.097	4
247		10	max	.002	1	.001	2	0	12	-1.432e-7	12	NC	1	NC	1
248			min	0	3	-.002	3	-.054	4	-7.218e-4	4	NC	1	458.783	4
249		11	max	.001	1	.001	2	0	12	-1.432e-7	12	NC	1	NC	1
250			min	0	3	-.002	3	-.044	4	-7.218e-4	4	NC	1	564.325	4
251		12	max	.001	1	.001	2	0	12	-1.432e-7	12	NC	1	NC	1
252			min	0	3	-.002	3	-.035	4	-7.218e-4	4	NC	1	715.488	4
253		13	max	.001	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
254			min	0	3	-.002	3	-.026	4	-7.218e-4	4	NC	1	943.393	4
255		14	max	0	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
256			min	0	3	-.001	3	-.019	4	-7.218e-4	4	NC	1	1311.364	4
257		15	max	0	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
258			min	0	3	-.001	3	-.013	4	-7.218e-4	4	NC	1	1965.401	4
259		16	max	0	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
260			min	0	3	0	3	-.007	4	-7.218e-4	4	NC	1	3309.201	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-7.218e-4	4	NC	1	6840.067	4
263		18	max	0	1	0	2	0	12	-1.432e-7	12	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-7.218e-4	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	-1.432e-7	12	NC	1	NC	1
266			min	0	1	0	1	0	1	-7.218e-4	4	NC	1	NC	1
267	M6	1	max	.017	1	.015	2	0	1	1.194e-3	4	NC	4	NC	1
268			min	-.02	3	-.023	3	-.41	4	0	1	2115.888	3	116.822	4
269		2	max	.016	1	.014	2	0	1	1.267e-3	4	NC	4	NC	1
270			min	-.019	3	-.021	3	-.376	4	0	1	2242.314	3	127.29	4
271		3	max	.015	1	.013	2	0	1	1.34e-3	4	NC	4	NC	1
272			min	-.018	3	-.02	3	-.342	4	0	1	2384.779	3	139.736	4
273		4	max	.014	1	.011	2	0	1	1.413e-3	4	NC	4	NC	1
274			min	-.017	3	-.019	3	-.309	4	0	1	2546.505	3	154.68	4
275		5	max	.013	1	.01	2	0	1	1.486e-3	4	NC	4	NC	1
276			min	-.016	3	-.018	3	-.277	4	0	1	2731.634	3	172.826	4
277		6	max	.012	1	.009	2	0	1	1.559e-3	4	NC	1	NC	1
278			min	-.015	3	-.016	3	-.245	4	0	1	2945.583	3	195.153	4
279		7	max	.011	1	.008	2	0	1	1.631e-3	4	NC	1	NC	1
280			min	-.014	3	-.015	3	-.215	4	0	1	3195.576	3	223.053	4
281		8	max	.011	1	.007	2	0	1	1.704e-3	4	NC	1	NC	1
282			min	-.013	3	-.014	3	-.185	4	0	1	3491.465	3	258.56	4
283		9	max	.01	1	.006	2	0	1	1.777e-3	4	NC	1	NC	1
284			min	-.011	3	-.012	3	-.157	4	0	1	3847.042	3	304.738	4
285		10	max	.009	1	.005	2	0	1	1.85e-3	4	NC	1	NC	1
286			min	-.01	3	-.011	3	-.131	4	0	1	4282.229	3	366.393	4
287		11	max	.008	1	.004	2	0	1	1.923e-3	4	NC	1	NC	1
288			min	-.009	3	-.01	3	-.106	4	0	1	4826.912	3	451.408	4
289		12	max	.007	1	.003	2	0	1	1.996e-3	4	NC	1	NC	1
290			min	-.008	3	-.009	3	-.083	4	0	1	5528.054	3	573.47	4
291		13	max	.006	1	.002	2	0	1	2.069e-3	4	NC	1	NC	1
292			min	-.007	3	-.007	3	-.063	4	0	1	6463.93	3	758.094	4
293		14	max	.005	1	.002	2	0	1	2.141e-3	4	NC	1	NC	1
294			min	-.006	3	-.006	3	-.045	4	0	1	7775.434	3	1057.499	4
295		15	max	.004	1	.001	2	0	1	2.214e-3	4	NC	1	NC	1
296			min	-.005	3	-.005	3	-.03	4	0	1	9744.36	3	1593.032	4
297		16	max	.003	1	0	2	0	1	2.287e-3	4	NC	1	NC	1
298			min	-.003	3	-.004	3	-.018	4	0	1	NC	1	2704.102	4
299		17	max	.002	1	0	2	0	1	2.36e-3	4	NC	1	NC	1
300			min	-.002	3	-.002	3	-.008	4	0	1	NC	1	5673.464	4
301		18	max	0	1	0	2	0	1	2.433e-3	4	NC	1	NC	1
302			min	-.001	3	-.001	3	-.002	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.506e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.577e-4	4	NC	1	NC	1
307		2	max	0	3	0	2	.012	4	1.369e-6	14	NC	1	NC	1
308			min	0	2	-.002	3	0	1	-1.173e-7	15	NC	1	NC	1
309		3	max	.002	3	0	15	.024	4	5.6e-4	4	NC	1	NC	1
310			min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
311		4	max	.002	3	-.001	15	.035	4	1.119e-3	4	NC	1	NC	1
312			min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
313		5	max	.003	3	-.002	15	.045	4	1.678e-3	4	NC	1	NC	1
314			min	-.003	2	-.008	3	0	1	0	1	NC	1	NC	1
315		6	max	.004	3	-.002	15	.056	4	2.236e-3	4	NC	1	NC	1
316			min	-.004	2	-.01	3	0	1	0	1	9425.887	3	9290.565	4
317		7	max	.005	3	-.002	15	.065	4	2.795e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.004	2	-.011	3	0	1	0	1	8382.943	3	8903.138	4
319	8	max	.006	3	-.003	15	.074	4	3.354e-3	4	NC	1	NC	1
320		min	-.005	2	-.012	3	0	1	0	1	7760.237	3	8928.465	4
321	9	max	.006	3	-.003	15	.083	4	3.913e-3	4	NC	1	NC	1
322		min	-.006	2	-.012	3	0	1	0	1	7430.059	3	9330.935	4
323	10	max	.007	3	-.003	15	.091	4	4.472e-3	4	NC	1	NC	1
324		min	-.007	2	-.013	4	0	1	0	1	7334.341	3	NC	1
325	11	max	.008	3	-.003	15	.099	4	5.031e-3	4	NC	1	NC	1
326		min	-.007	2	-.013	4	0	1	0	1	7456.996	3	NC	1
327	12	max	.009	3	-.003	15	.107	4	5.589e-3	4	NC	1	NC	1
328		min	-.008	2	-.013	4	0	1	0	1	7665.245	4	NC	1
329	13	max	.01	3	-.003	15	.115	4	6.148e-3	4	NC	1	NC	1
330		min	-.009	2	-.012	4	0	1	0	1	8164.226	4	NC	1
331	14	max	.01	3	-.003	15	.122	4	6.707e-3	4	NC	1	NC	1
332		min	-.01	2	-.011	1	0	1	0	1	9078.748	4	NC	1
333	15	max	.011	3	-.002	15	.13	4	7.266e-3	4	NC	1	NC	1
334		min	-.01	2	-.011	1	0	1	0	1	NC	1	NC	1
335	16	max	.012	3	-.002	15	.138	4	7.825e-3	4	NC	1	NC	1
336		min	-.011	2	-.01	1	0	1	0	1	NC	1	NC	1
337	17	max	.013	3	-.001	15	.146	4	8.384e-3	4	NC	1	NC	1
338		min	-.012	2	-.009	1	0	1	0	1	NC	1	NC	1
339	18	max	.014	3	0	15	.155	4	8.942e-3	4	NC	1	NC	1
340		min	-.013	2	-.008	1	0	1	0	1	NC	1	NC	1
341	19	max	.014	3	0	15	.165	4	9.501e-3	4	NC	1	NC	1
342		min	-.013	2	-.007	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.011	2	0	0	1	NC	1	NC	1
344		min	-.003	3	-.014	3	-.165	4	-7.45e-4	4	NC	1	150.768	4
345	2	max	.009	1	.011	2	0	1	0	1	NC	1	NC	1
346		min	-.003	3	-.013	3	-.151	4	-7.45e-4	4	NC	1	164.13	4
347	3	max	.008	1	.01	2	0	1	0	1	NC	1	NC	1
348		min	-.003	3	-.013	3	-.138	4	-7.45e-4	4	NC	1	180.022	4
349	4	max	.008	1	.009	2	0	1	0	1	NC	1	NC	1
350		min	-.003	3	-.012	3	-.125	4	-7.45e-4	4	NC	1	199.103	4
351	5	max	.007	1	.009	2	0	1	0	1	NC	1	NC	1
352		min	-.003	3	-.011	3	-.112	4	-7.45e-4	4	NC	1	222.266	4
353	6	max	.007	1	.008	2	0	1	0	1	NC	1	NC	1
354		min	-.003	3	-.01	3	-.099	4	-7.45e-4	4	NC	1	250.755	4
355	7	max	.006	1	.008	2	0	1	0	1	NC	1	NC	1
356		min	-.002	3	-.009	3	-.087	4	-7.45e-4	4	NC	1	286.331	4
357	8	max	.006	1	.007	2	0	1	0	1	NC	1	NC	1
358		min	-.002	3	-.009	3	-.075	4	-7.45e-4	4	NC	1	331.566	4
359	9	max	.005	1	.006	2	0	1	0	1	NC	1	NC	1
360		min	-.002	3	-.008	3	-.064	4	-7.45e-4	4	NC	1	390.329	4
361	10	max	.005	1	.006	2	0	1	0	1	NC	1	NC	1
362		min	-.002	3	-.007	3	-.053	4	-7.45e-4	4	NC	1	468.672	4
363	11	max	.004	1	.005	2	0	1	0	1	NC	1	NC	1
364		min	-.002	3	-.006	3	-.043	4	-7.45e-4	4	NC	1	576.493	4
365	12	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
366		min	-.001	3	-.005	3	-.034	4	-7.45e-4	4	NC	1	730.922	4
367	13	max	.003	1	.004	2	0	1	0	1	NC	1	NC	1
368		min	-.001	3	-.005	3	-.026	4	-7.45e-4	4	NC	1	963.75	4
369	14	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.004	3	-.019	4	-7.45e-4	4	NC	1	1339.671	4
371	15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.003	3	-.012	4	-7.45e-4	4	NC	1	2007.841	4
373	16	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.007	4	-7.45e-4	4	NC	1	3380.684	4





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.004	4	-7.45e-4	4	NC	1	6987.888	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-7.45e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-7.45e-4	4	NC	1	NC	1
381	M10	1	max	.005	1	.004	2	0	12	1.193e-3	4	NC	1	NC	2
382			min	-.006	3	-.007	3	-.409	4	7.068e-6	12	NC	1	116.994	4
383		2	max	.005	1	.003	2	0	12	1.265e-3	4	NC	1	NC	2
384			min	-.006	3	-.007	3	-.375	4	6.576e-6	12	NC	1	127.477	4
385		3	max	.005	1	.003	2	0	12	1.338e-3	4	NC	1	NC	1
386			min	-.006	3	-.007	3	-.342	4	6.084e-6	12	NC	1	139.941	4
387		4	max	.004	1	.002	2	0	12	1.41e-3	4	NC	1	NC	1
388			min	-.005	3	-.006	3	-.309	4	5.592e-6	12	NC	1	154.907	4
389		5	max	.004	1	.002	2	0	12	1.483e-3	4	NC	1	NC	1
390			min	-.005	3	-.006	3	-.276	4	5.1e-6	12	NC	1	173.08	4
391		6	max	.004	1	.001	2	0	12	1.556e-3	4	NC	1	NC	1
392			min	-.005	3	-.006	3	-.245	4	4.608e-6	12	NC	1	195.44	4
393		7	max	.004	1	0	2	0	12	1.628e-3	4	NC	1	NC	1
394			min	-.004	3	-.005	3	-.214	4	4.116e-6	12	NC	1	223.381	4
395		8	max	.003	1	0	2	0	12	1.701e-3	4	NC	1	NC	1
396			min	-.004	3	-.005	3	-.185	4	3.624e-6	12	NC	1	258.939	4
397		9	max	.003	1	0	2	0	12	1.774e-3	4	NC	1	NC	1
398			min	-.003	3	-.005	3	-.157	4	3.132e-6	12	NC	1	305.185	4
399		10	max	.003	1	0	2	0	12	1.846e-3	4	NC	1	NC	1
400			min	-.003	3	-.004	3	-.13	4	2.64e-6	12	NC	1	366.931	4
401		11	max	.002	1	0	2	0	12	1.919e-3	4	NC	1	NC	1
402			min	-.003	3	-.004	3	-.106	4	2.148e-6	12	NC	1	452.072	4
403		12	max	.002	1	0	2	0	12	1.991e-3	4	NC	1	NC	1
404			min	-.002	3	-.004	3	-.083	4	1.656e-6	12	NC	1	574.314	4
405		13	max	.002	1	0	15	0	12	2.064e-3	4	NC	1	NC	1
406			min	-.002	3	-.003	3	-.063	4	1.155e-6	10	NC	1	759.211	4
407		14	max	.001	1	0	15	0	12	2.137e-3	4	NC	1	NC	1
408			min	-.002	3	-.003	3	-.045	4	3.515e-7	10	NC	1	1059.06	4
409		15	max	.001	1	0	15	0	12	2.209e-3	4	NC	1	NC	1
410			min	-.001	3	-.002	3	-.03	4	-3.629e-6	1	NC	1	1595.389	4
411		16	max	0	1	0	15	0	12	2.282e-3	4	NC	1	NC	1
412			min	-.001	3	-.002	3	-.018	4	-1.372e-5	1	NC	1	2708.123	4
413		17	max	0	1	0	15	0	12	2.354e-3	4	NC	1	NC	1
414			min	0	3	-.001	4	-.008	4	-2.38e-5	1	NC	1	5681.985	4
415		18	max	0	1	0	15	0	12	2.427e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.002	4	-3.389e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.5e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-4.398e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.368e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-5.562e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.012	4	4.766e-6	4	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-3.725e-6	1	NC	1	NC	1
423		3	max	0	3	0	15	.024	4	5.657e-4	4	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.113e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.035	4	1.127e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-3.853e-5	1	NC	1	NC	1
427		5	max	.001	3	-.002	15	.045	4	1.688e-3	4	NC	1	NC	1
428			min	0	2	-.007	4	0	1	-5.594e-5	1	NC	1	NC	1
429		6	max	.001	3	-.002	15	.055	4	2.249e-3	4	NC	1	NC	1
430			min	0	2	-.009	4	-.001	1	-7.334e-5	1	NC	1	9643.446	4
431		7	max	.002	3	-.003	15	.065	4	2.81e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.001	2	-.01	4	-.001	1	-9.075e-5	1	9107.415	4	9278.164	4
433		8	max	.002	3	-.003	15	.074	4	3.371e-3	4	NC	1	NC	1
434			min	-.001	2	-.012	4	-.002	1	-1.082e-4	1	8130.599	4	9349.586	4
435		9	max	.002	3	-.003	15	.083	4	3.932e-3	4	NC	1	NC	1
436			min	-.002	2	-.013	4	-.002	1	-1.256e-4	1	7548.025	4	9829.718	4
437		10	max	.002	3	-.003	15	.091	4	4.493e-3	4	NC	2	NC	1
438			min	-.002	2	-.013	4	-.002	1	-1.43e-4	1	7256.642	4	NC	1
439		11	max	.003	3	-.003	15	.099	4	5.054e-3	4	NC	2	NC	1
440			min	-.002	2	-.013	4	-.003	1	-1.604e-4	1	7212.947	4	NC	1
441		12	max	.003	3	-.003	15	.107	4	5.615e-3	4	NC	1	NC	1
442			min	-.002	2	-.013	4	-.003	1	-1.778e-4	1	7416.211	4	NC	1
443		13	max	.003	3	-.003	15	.115	4	6.176e-3	4	NC	1	NC	1
444			min	-.002	2	-.012	4	-.003	1	-1.952e-4	1	7910.264	4	NC	1
445		14	max	.003	3	-.003	15	.123	4	6.736e-3	4	NC	1	NC	1
446			min	-.003	2	-.011	4	-.003	1	-2.126e-4	1	8806.728	4	NC	1
447		15	max	.004	3	-.002	15	.131	4	7.297e-3	4	NC	1	NC	1
448			min	-.003	2	-.01	4	-.004	1	-2.3e-4	1	NC	1	NC	1
449		16	max	.004	3	-.002	15	.139	4	7.858e-3	4	NC	1	NC	1
450			min	-.003	2	-.008	4	-.004	1	-2.474e-4	1	NC	1	NC	1
451		17	max	.004	3	-.001	15	.147	4	8.419e-3	4	NC	1	NC	1
452			min	-.003	2	-.006	1	-.004	1	-2.648e-4	1	NC	1	NC	1
453		18	max	.004	3	0	15	.156	4	8.98e-3	4	NC	1	NC	1
454			min	-.003	2	-.005	1	-.005	1	-2.822e-4	1	NC	1	NC	1
455		19	max	.005	3	0	12	.165	4	9.541e-3	4	NC	1	NC	1
456			min	-.004	2	-.003	1	-.005	1	-2.996e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.003	2	.005	1	8.018e-6	1	NC	1	NC	2
458			min	-.001	3	-.005	3	-.165	4	-7.177e-4	4	NC	1	149.957	4
459		2	max	.003	1	.003	2	.005	1	8.018e-6	1	NC	1	NC	2
460			min	0	3	-.004	3	-.152	4	-7.177e-4	4	NC	1	163.245	4
461		3	max	.003	1	.003	2	.004	1	8.018e-6	1	NC	1	NC	2
462			min	0	3	-.004	3	-.139	4	-7.177e-4	4	NC	1	179.047	4
463		4	max	.003	1	.002	2	.004	1	8.018e-6	1	NC	1	NC	2
464			min	0	3	-.004	3	-.125	4	-7.177e-4	4	NC	1	198.02	4
465		5	max	.002	1	.002	2	.004	1	8.018e-6	1	NC	1	NC	2
466			min	0	3	-.004	3	-.112	4	-7.177e-4	4	NC	1	221.054	4
467		6	max	.002	1	.002	2	.003	1	8.018e-6	1	NC	1	NC	2
468			min	0	3	-.003	3	-.099	4	-7.177e-4	4	NC	1	249.383	4
469		7	max	.002	1	.002	2	.003	1	8.018e-6	1	NC	1	NC	2
470			min	0	3	-.003	3	-.087	4	-7.177e-4	4	NC	1	284.76	4
471		8	max	.002	1	.002	2	.002	1	8.018e-6	1	NC	1	NC	1
472			min	0	3	-.003	3	-.075	4	-7.177e-4	4	NC	1	329.741	4
473		9	max	.002	1	.002	2	.002	1	8.018e-6	1	NC	1	NC	1
474			min	0	3	-.003	3	-.064	4	-7.177e-4	4	NC	1	388.174	4
475		10	max	.002	1	.001	2	.002	1	8.018e-6	1	NC	1	NC	1
476			min	0	3	-.002	3	-.053	4	-7.177e-4	4	NC	1	466.078	4
477		11	max	.001	1	.001	2	.001	1	8.018e-6	1	NC	1	NC	1
478			min	0	3	-.002	3	-.043	4	-7.177e-4	4	NC	1	573.294	4
479		12	max	.001	1	.001	2	.001	1	8.018e-6	1	NC	1	NC	1
480			min	0	3	-.002	3	-.034	4	-7.177e-4	4	NC	1	726.856	4
481		13	max	.001	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
482			min	0	3	-.002	3	-.026	4	-7.177e-4	4	NC	1	958.376	4
483		14	max	0	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
484			min	0	3	-.001	3	-.019	4	-7.177e-4	4	NC	1	1332.183	4
485		15	max	0	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
486			min	0	3	-.001	3	-.012	4	-7.177e-4	4	NC	1	1996.591	4
487		16	max	0	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
488			min	0	3	0	3	-.007	4	-7.177e-4	4	NC	1	3361.695	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-7.177e-4	4	NC	1	6948.522	4
491		18	max	0	1	0	2	0	1	8.018e-6	1	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-7.177e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	8.018e-6	1	NC	1	NC	1
494			min	0	1	0	1	0	1	-7.177e-4	4	NC	1	NC	1
495	M1	1	max	.006	3	.127	2	.428	4	1.486e-2	1	NC	1	NC	1
496			min	-.003	2	-.029	3	0	12	-2.408e-2	3	NC	1	NC	1
497		2	max	.006	3	.062	2	.417	4	7.594e-3	4	NC	4	NC	1
498			min	-.003	2	-.015	3	-.004	1	-1.191e-2	3	1781.638	2	NC	1
499		3	max	.006	3	.008	3	.406	4	1.246e-2	4	NC	5	NC	1
500			min	-.003	2	-.007	2	-.005	1	-1.193e-4	3	859.051	2	9547.713	5
501		4	max	.005	3	.046	3	.395	4	1.094e-2	4	NC	5	NC	1
502			min	-.003	2	-.086	2	-.005	1	-4.667e-3	3	542.619	2	6494.483	5
503		5	max	.005	3	.094	3	.385	4	9.501e-3	1	NC	15	NC	1
504			min	-.003	2	-.167	2	-.004	1	-9.215e-3	3	391.82	2	4956.033	5
505		6	max	.005	3	.145	3	.374	4	1.429e-2	1	NC	15	NC	1
506			min	-.002	2	-.248	1	-.001	1	-1.376e-2	3	307.862	1	4052.109	5
507		7	max	.005	3	.194	3	.362	4	1.909e-2	1	NC	15	NC	1
508			min	-.002	2	-.32	1	0	12	-1.831e-2	3	258.014	1	3468.989	4
509		8	max	.005	3	.235	3	.35	4	2.388e-2	1	9018.998	15	NC	1
510			min	-.002	2	-.377	1	0	12	-2.286e-2	3	228.604	1	3075.374	4
511		9	max	.005	3	.262	3	.337	4	2.64e-2	1	8432.713	15	NC	1
512			min	-.002	2	-.413	1	0	1	-2.312e-2	3	213.32	1	2849.154	4
513		10	max	.005	3	.272	3	.321	4	2.741e-2	1	8254.034	15	NC	1
514			min	-.002	2	-.425	1	0	12	-2.053e-2	3	208.759	1	2784.819	4
515		11	max	.005	3	.265	3	.304	4	2.922e-2	2	8432.524	15	NC	1
516			min	-.002	2	-.413	1	0	12	-1.795e-2	3	213.663	1	2852.453	4
517		12	max	.005	3	.243	3	.285	4	2.82e-2	2	9018.562	15	NC	1
518			min	-.002	2	-.376	1	0	1	-1.519e-2	3	229.656	1	3068.463	5
519		13	max	.005	3	.206	3	.263	4	2.261e-2	2	NC	15	NC	1
520			min	-.002	2	-.317	1	0	1	-1.216e-2	3	260.589	1	3611.415	4
521		14	max	.004	3	.16	3	.239	4	1.702e-2	2	NC	15	NC	1
522			min	-.002	2	-.244	1	0	12	-9.131e-3	3	313.37	1	4723.335	4
523		15	max	.004	3	.108	3	.215	4	1.143e-2	2	NC	15	NC	1
524			min	-.002	2	-.163	1	0	12	-6.104e-3	3	403.933	1	7102.538	4
525		16	max	.004	3	.055	3	.191	4	8.391e-3	4	NC	5	NC	1
526			min	-.002	2	-.081	1	0	12	-3.077e-3	3	570.913	1	NC	1
527		17	max	.004	3	.003	3	.169	4	9.325e-3	4	NC	5	NC	1
528			min	-.002	2	-.005	2	0	12	-4.932e-5	3	926.314	1	NC	1
529		18	max	.004	3	.06	1	.149	4	9.932e-3	2	NC	4	NC	1
530			min	-.002	2	-.042	3	0	12	-4.186e-3	3	1955.565	1	NC	1
531		19	max	.004	3	.118	1	.133	4	1.998e-2	2	NC	1	NC	1
532			min	-.002	2	-.085	3	0	1	-8.49e-3	3	NC	1	NC	1
533	M5	1	max	.017	3	.29	2	.428	4	0	1	NC	1	NC	1
534			min	-.011	2	-.04	3	0	1	-2.477e-6	4	NC	1	NC	1
535		2	max	.017	3	.142	2	.419	4	6.377e-3	4	NC	5	NC	1
536			min	-.011	2	-.02	3	0	1	0	1	786.654	2	NC	1
537		3	max	.017	3	.027	3	.409	4	1.256e-2	4	NC	5	NC	1
538			min	-.011	2	-.023	2	0	1	0	1	370.51	2	8025.488	4
539		4	max	.017	3	.123	3	.398	4	1.023e-2	4	NC	15	NC	1
540			min	-.011	2	-.22	2	0	1	0	1	227.109	2	5817.073	4
541		5	max	.017	3	.254	3	.387	4	7.905e-3	4	7140.443	15	NC	1
542			min	-.011	2	-.434	1	0	1	0	1	160.037	2	4681.437	4
543		6	max	.016	3	.4	3	.375	4	5.579e-3	4	5494.436	15	NC	1
544			min	-.011	2	-.649	1	0	1	0	1	123.358	1	3979.642	4
545		7	max	.016	3	.541	3	.362	4	3.253e-3	4	4544.471	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.01	2	-.844	1	0	1	0	1	101.952	1	3489.202	4
547		8	max	.016	3	.66	3	.35	4	9.269e-4	4	3992.379	15	NC	1
548			min	-.01	2	-1	1	0	1	0	1	89.508	1	3111.588	4
549		9	max	.015	3	.736	3	.337	4	0	1	3709.351	15	NC	1
550			min	-.01	2	-1.099	1	0	1	-1.435e-6	5	83.132	1	2849.551	4
551		10	max	.015	3	.764	3	.321	4	0	1	3624.083	15	NC	1
552			min	-.01	2	-1.132	1	0	1	-1.358e-6	5	81.24	1	2805.541	4
553		11	max	.015	3	.745	3	.304	4	0	1	3709.426	15	NC	1
554			min	-.01	2	-1.099	1	0	1	-1.28e-6	5	83.274	1	2881.504	4
555		12	max	.014	3	.68	3	.286	4	6.719e-4	4	3992.555	15	NC	1
556			min	-.009	2	-.998	1	0	1	0	1	89.976	1	3026.698	4
557		13	max	.014	3	.576	3	.264	4	2.358e-3	4	4544.827	15	NC	1
558			min	-.009	2	-.837	1	0	1	0	1	103.17	1	3552.584	4
559		14	max	.014	3	.444	3	.239	4	4.044e-3	4	5495.128	15	NC	1
560			min	-.009	2	-.638	1	0	1	0	1	126.106	1	4833.285	4
561		15	max	.013	3	.297	3	.213	4	5.731e-3	4	7141.804	15	NC	1
562			min	-.009	2	-.42	1	0	1	0	1	166.409	1	8145.11	5
563		16	max	.013	3	.148	3	.188	4	7.417e-3	4	NC	15	NC	1
564			min	-.009	2	-.205	1	0	1	0	1	243.018	1	NC	1
565		17	max	.013	3	.01	3	.165	4	9.103e-3	4	NC	5	NC	1
566			min	-.009	2	-.015	2	0	1	0	1	411.348	1	NC	1
567		18	max	.013	3	.135	1	.147	4	4.623e-3	4	NC	5	NC	1
568			min	-.009	2	-.107	3	0	1	0	1	898.332	1	NC	1
569		19	max	.013	3	.261	1	.133	4	0	1	NC	1	NC	1
570			min	-.009	2	-.212	3	0	1	-1.018e-6	4	NC	1	NC	1
571	M9	1	max	.006	3	.127	2	.428	4	2.408e-2	3	NC	1	NC	1
572			min	-.003	2	-.029	3	0	1	-1.486e-2	1	NC	1	NC	1
573		2	max	.006	3	.062	2	.419	4	1.191e-2	3	NC	4	NC	1
574			min	-.003	2	-.015	3	0	12	-7.245e-3	1	1781.638	2	NC	1
575		3	max	.006	3	.008	3	.409	4	1.252e-2	4	NC	5	NC	1
576			min	-.003	2	-.007	2	0	12	-3.501e-5	10	859.051	2	8248.466	4
577		4	max	.005	3	.046	3	.398	4	9.856e-3	5	NC	5	NC	1
578			min	-.003	2	-.086	2	0	12	-4.707e-3	1	542.619	2	5904.595	4
579		5	max	.005	3	.094	3	.386	4	9.215e-3	3	NC	15	NC	1
580			min	-.003	2	-.167	2	0	12	-9.501e-3	1	391.82	2	4701.388	4
581		6	max	.005	3	.145	3	.375	4	1.376e-2	3	NC	15	NC	1
582			min	-.002	2	-.248	1	0	12	-1.429e-2	1	307.862	1	3966.649	4
583		7	max	.005	3	.194	3	.362	4	1.831e-2	3	NC	15	NC	1
584			min	-.002	2	-.32	1	0	1	-1.909e-2	1	258.014	1	3466.124	4
585		8	max	.005	3	.235	3	.35	4	2.286e-2	3	9007.442	15	NC	1
586			min	-.002	2	-.377	1	0	1	-2.388e-2	1	228.604	1	3093.65	5
587		9	max	.005	3	.262	3	.337	4	2.312e-2	3	8422.029	15	NC	1
588			min	-.002	2	-.413	1	0	12	-2.64e-2	1	213.32	1	2843.131	4
589		10	max	.005	3	.272	3	.321	4	2.053e-2	3	8243.6	15	NC	1
590			min	-.002	2	-.425	1	0	1	-2.741e-2	1	208.759	1	2785.625	4
591		11	max	.005	3	.265	3	.304	4	1.795e-2	3	8421.832	15	NC	1
592			min	-.002	2	-.413	1	0	1	-2.922e-2	2	213.663	1	2859.761	4
593		12	max	.005	3	.243	3	.285	4	1.519e-2	3	9007.059	15	NC	1
594			min	-.002	2	-.376	1	0	12	-2.82e-2	2	229.656	1	3050.032	4
595		13	max	.005	3	.206	3	.263	4	1.216e-2	3	NC	15	NC	1
596			min	-.002	2	-.317	1	0	12	-2.261e-2	2	260.589	1	3610.862	4
597		14	max	.004	3	.16	3	.238	4	9.131e-3	3	NC	15	NC	1
598			min	-.002	2	-.244	1	-.001	1	-1.702e-2	2	313.37	1	4820.835	5
599		15	max	.004	3	.108	3	.213	4	6.104e-3	3	NC	15	NC	1
600			min	-.002	2	-.163	1	-.003	1	-1.143e-2	2	403.933	1	7600.237	5
601		16	max	.004	3	.055	3	.188	4	7.237e-3	5	NC	5	NC	1
602			min	-.002	2	-.081	1	-.005	1	-5.832e-3	2	570.913	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.004	3	.003	3	.166	4	9.14e-3	4	NC	5	NC	1
604		min	-.002	2	-.005	2	-.005	1	-4.173e-4	1	926.314	1	NC	1
605	18	max	.004	3	.06	1	.148	4	4.357e-3	5	NC	4	NC	1
606		min	-.002	2	-.042	3	-.004	1	-9.932e-3	2	1955.565	1	NC	1
607	19	max	.004	3	.118	1	.133	4	8.49e-3	3	NC	1	NC	1
608		min	-.002	2	-.085	3	0	12	-1.998e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

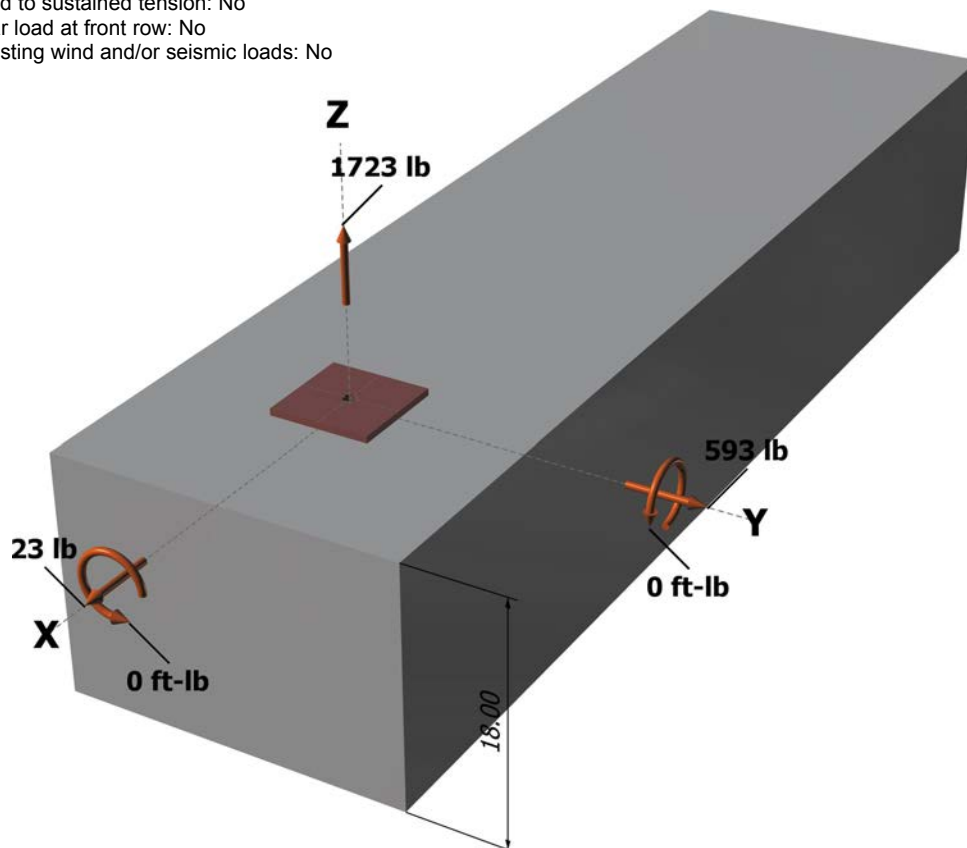
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298





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Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 34-35 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

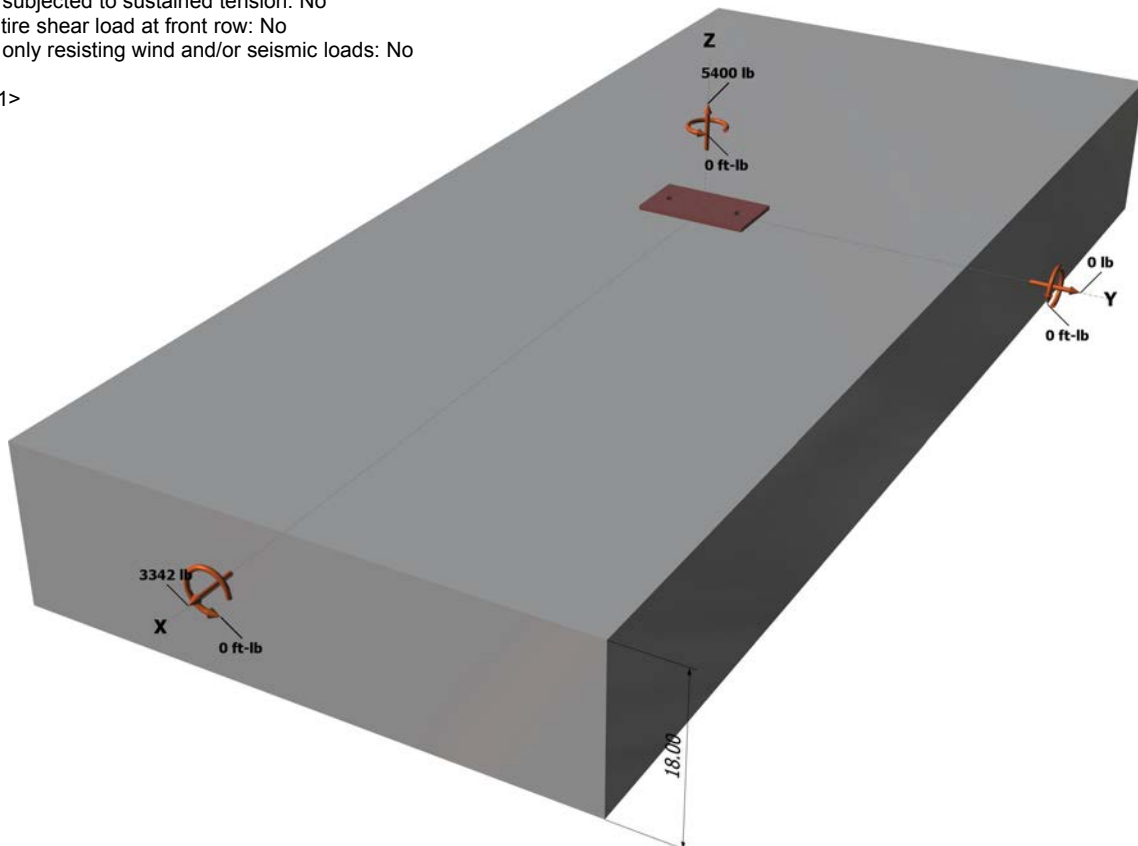
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

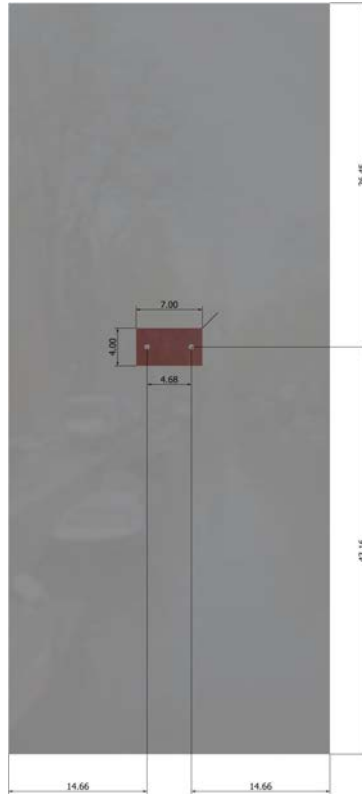
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

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Address:			
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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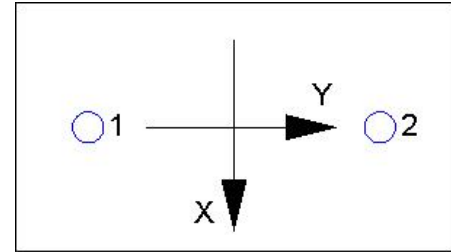
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2700.0	1671.0	0.0	1671.0
2	2700.0	1671.0	0.0	1671.0
Sum	5400.0	3342.0	0.0	3342.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5400  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
612.00	648.00	1.000	0.944	1.000	1.000	15593	0.70	9735

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c} c_{at}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	14.66	21056

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
791.64	967.12	1.000	1.000	1.000	21056	0.70	24129

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2700	6071	0.44	Pass	
Concrete breakout	5400	10231	0.53	Pass	
<b>Adhesive</b>	<b>5400</b>	<b>8093</b>	<b>0.67</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1671</b>	<b>3156</b>	<b>0.53</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3342	9735	0.34	Pass	
Concrete breakout y-	1671	24129	0.07	Pass	
Pryout	3342	20601	0.16	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.67	0.53	119.7 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.