



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

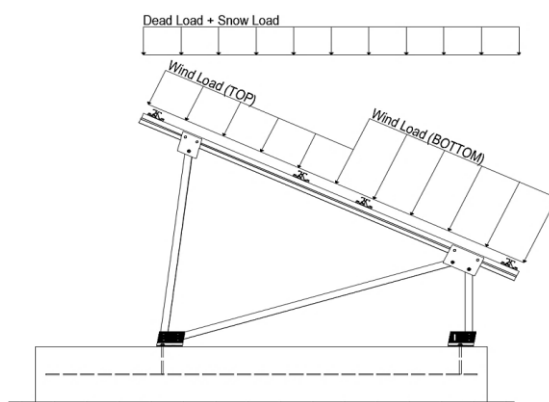
$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.07	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	93 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.684 k-ft
$M_z$ =	0.110 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>70%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.285 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.991 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

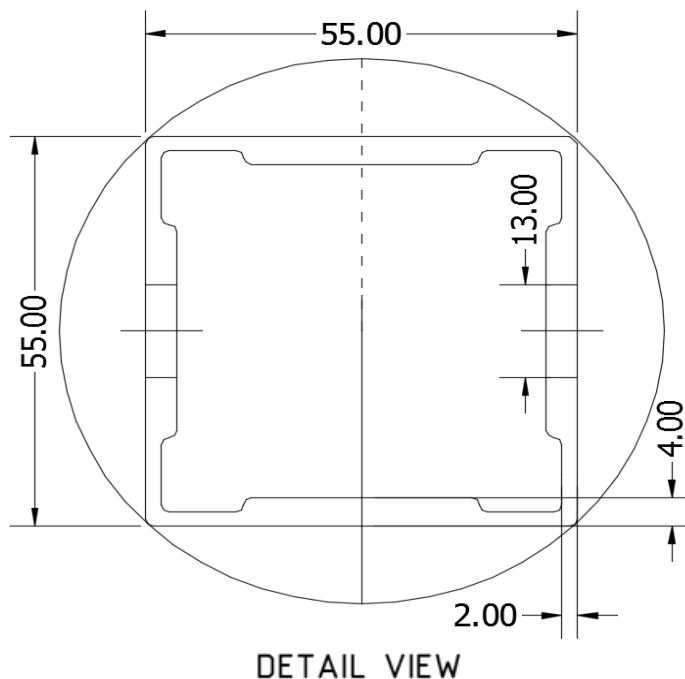
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.513 k-ft
$P_n$ =	0.379 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>38%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.639 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>45%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.216 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<b>38%</b>



#### 5. FOUNDATION DESIGN CALCULATIONS

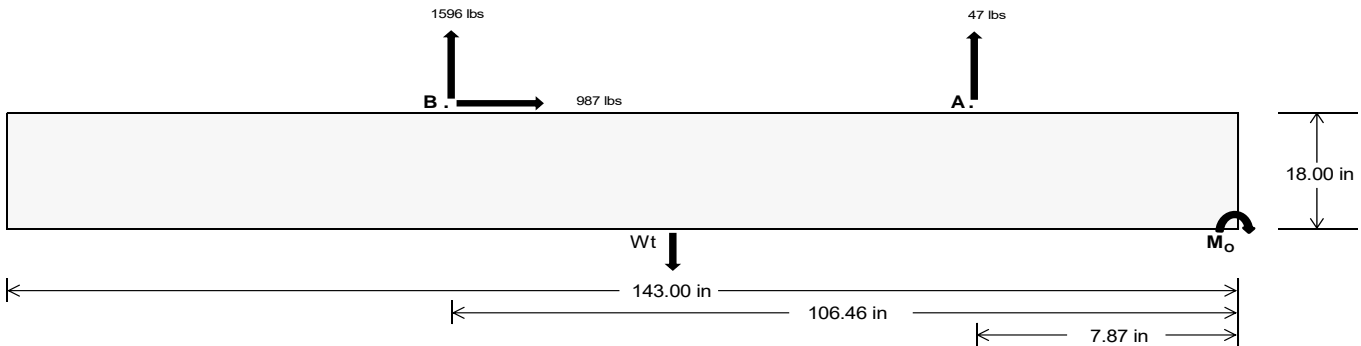
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>206.57</u>	<u>6649.22</u> k
Compressive Load =		<u>3260.13</u>	<u>5004.81</u> k
Lateral Load =		<u>350.93</u>	<u>4105.58</u> k
Moment (Weak Axis) =		<u>0.68</u>	<u>0.24</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 188083.4$  in-lbs  
Resisting Force Required = 2630.54 lbs  
S.F. = 1.67  
Weight Required = 4384.23 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Sliding

Force = 987.09 lbs  
Friction = 0.4  
Weight Required = 2467.73 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 987.09 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1087 lbs	1087 lbs	1087 lbs	1087 lbs	1309 lbs	1309 lbs	1309 lbs	1309 lbs	1682 lbs	1682 lbs	1682 lbs	1682 lbs	-94 lbs	-94 lbs	-94 lbs	-94 lbs
$F_B$	1066 lbs	1066 lbs	1066 lbs	1066 lbs	2198 lbs	2198 lbs	2198 lbs	2198 lbs	2336 lbs	2336 lbs	2336 lbs	2336 lbs	-3193 lbs	-3193 lbs	-3193 lbs	-3193 lbs
$F_V$	142 lbs	142 lbs	142 lbs	142 lbs	1777 lbs	1777 lbs	1777 lbs	1777 lbs	1426 lbs	1426 lbs	1426 lbs	1426 lbs	-1974 lbs	-1974 lbs	-1974 lbs	-1974 lbs
$P_{total}$	9713 lbs	9929 lbs	10145 lbs	10361 lbs	11067 lbs	11283 lbs	11499 lbs	11715 lbs	11577 lbs	11793 lbs	12009 lbs	12225 lbs	1249 lbs	1379 lbs	1508 lbs	1638 lbs
$M$	2872 lbs-ft	2872 lbs-ft	2872 lbs-ft	2872 lbs-ft	3203 lbs-ft	3203 lbs-ft	3203 lbs-ft	3203 lbs-ft	4250 lbs-ft	4250 lbs-ft	4250 lbs-ft	4250 lbs-ft	5842 lbs-ft	5842 lbs-ft	5842 lbs-ft	5842 lbs-ft
$e$	0.30 ft	0.29 ft	0.28 ft	0.28 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.37 ft	0.36 ft	0.35 ft	0.35 ft	4.68 ft	4.24 ft	3.87 ft	3.57 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	237.8 psf	237.3 psf	236.7 psf	236.2 psf	272.0 psf	270.5 psf	269.1 psf	267.7 psf	271.5 psf	270.0 psf	268.6 psf	267.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	321.1 psf	318.2 psf	315.5 psf	312.9 psf	364.8 psf	360.7 psf	356.9 psf	353.2 psf	394.7 psf	389.7 psf	385.1 psf	380.7 psf	222.8 psf	178.0 psf	156.4 psf	144.2 psf

Maximum Bearing Pressure = 395 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

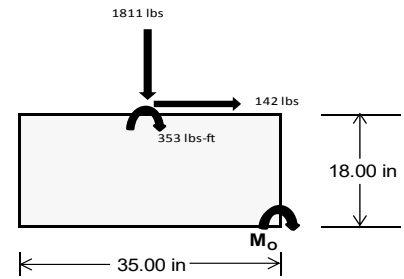
### Overturning Check

$M_o = 2076.0$  ft-lbs  
 Resisting Force Required = 1423.52 lbs  
 S.F. = 1.67  
 Weight Required = 2372.53 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	304 lbs	566 lbs	173 lbs	731 lbs	1811 lbs	630 lbs	135 lbs	165 lbs	4 lbs
$F_h$	197 lbs	192 lbs	201 lbs	144 lbs	142 lbs	156 lbs	198 lbs	193 lbs	199 lbs
$P_{total}$	9663 lbs	9925 lbs	9532 lbs	9640 lbs	10720 lbs	9539 lbs	2872 lbs	2902 lbs	2741 lbs
$M$	763 lbs-ft	750 lbs-ft	774 lbs-ft	566 lbs-ft	565 lbs-ft	605 lbs-ft	764 lbs-ft	747 lbs-ft	768 lbs-ft
$e$	0.08 ft	0.08 ft	0.08 ft	0.06 ft	0.05 ft	0.06 ft	0.27 ft	0.26 ft	0.28 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	232.8 psf	241.2 psf	228.4 psf	243.8 psf	275.0 psf	238.6 psf	37.4 psf	39.3 psf	33.4 psf
$f_{max}$	323.2 psf	329.9 psf	320.1 psf	310.9 psf	341.9 psf	310.3 psf	127.8 psf	127.7 psf	124.3 psf



Maximum Bearing Pressure = 342 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 34in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.875 k
Allowable Uplift =	1.214 k
Utilization =	<u>72%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.425 k
Allowable Uplift =	4.357 k
Utilization =	<u>56%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.508 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>34%</u>

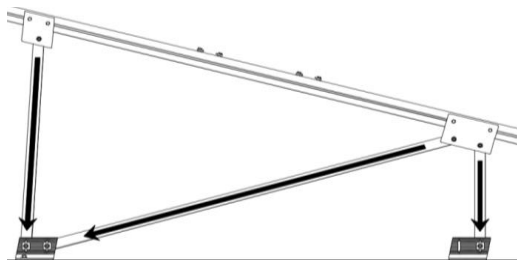
#### Rear Strut

Maximum Axial Load =	4.475 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>60%</u>

#### Diagonal Strut

Maximum Axial Load =	2.775 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>37%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	60.93 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.219 in
	<u>0.7 ≤ 1.219, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 93 \text{ in}$$

$$J = 0.432$$

$$257.282$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 28.1 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 93$$

$$J = 0.432$$

$$163.616$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.2$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.7

$$\begin{aligned} \lambda &= 1.8125 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83375 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 8.88278 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.88 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.14 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.866	-46.866	0	0
2	M14	Y	-46.866	-46.866	0	0
3	M15	Y	-46.866	-46.866	0	0
4	M16	Y	-46.866	-46.866	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-71.679	-71.679	0	0
2	M14	y	-71.679	-71.679	0	0
3	M15	y	-115.31	-115.31	0	0
4	M16	y	-115.31	-115.31	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	162.058	162.058	0	0
2	M14	y	124.66	124.66	0	0
3	M15	y	68.563	68.563	0	0
4	M16	y	68.563	68.563	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	61.75	4	722.226	2	-6.58	12	.014	2	.25	1	1.19	2
20			min	5.418	12	-1225.914	3	-181.797	1	-.004	3	.004	12	-1.877	3
21		11	max	61.262	1	595.839	2	-4.782	12	.014	2	.121	4	.622	2
22			min	5.418	12	-1007.304	3	-143.648	1	0	15	-.003	3	-.915	3
23		12	max	61.262	1	469.452	2	-2.984	12	.014	2	.061	4	.163	2
24			min	5.418	12	-788.695	3	-105.5	1	0	15	-.007	3	-.142	3
25		13	max	61.262	1	343.065	2	-1.187	12	.014	2	.029	5	.443	3
26			min	5.418	12	-570.085	3	-67.352	1	0	15	-.072	1	-.186	2
27		14	max	61.262	1	216.678	2	1.104	3	.014	2	0	15	.84	3
28			min	4.532	15	-351.475	3	-40.004	4	0	15	-.114	1	-.427	2
29		15	max	61.262	1	90.29	2	8.944	1	.014	2	-.005	12	1.048	3
30			min	-3.31	5	-132.865	3	-30.283	5	0	15	-.122	1	-.56	2
31		16	max	61.262	1	85.745	3	47.092	1	.014	2	-.003	12	1.069	3
32			min	-13.471	5	-36.097	2	-27.547	5	0	15	-.098	1	-.583	2
33		17	max	61.262	1	304.355	3	85.24	1	.014	2	.003	3	.901	3
34			min	-23.633	5	-162.484	2	-24.811	5	0	15	-.086	4	-.497	2
35		18	max	61.262	1	522.964	3	123.389	1	.014	2	.049	1	.544	3
36			min	-33.794	5	-288.871	2	-22.075	5	0	15	-.096	5	-.303	2
37		19	max	61.262	1	741.574	3	161.537	1	.014	2	.171	1	0	2
38			min	-43.956	5	-415.259	2	-19.339	5	0	15	-.114	5	0	3
39	M14	1	max	42.152	4	487.109	2	-9.969	12	.014	3	.266	4	0	4
40			min	2.908	12	-611.591	3	-168.565	1	-.015	2	.018	12	0	3
41		2	max	39.025	1	360.722	2	-8.172	12	.014	3	.182	4	.455	3
42			min	2.908	12	-444.507	3	-130.417	1	-.015	2	.006	10	-.365	2
43		3	max	39.025	1	234.335	2	-6.374	12	.014	3	.11	5	.766	3
44			min	2.908	12	-277.424	3	-92.269	1	-.015	2	-.017	1	-.621	2
45		4	max	39.025	1	107.947	2	-4.576	12	.014	3	.062	5	.932	3
46			min	1.772	15	-110.34	3	-68.297	4	-.015	2	-.08	1	-.769	2
47		5	max	39.025	1	56.744	3	-.679	10	.014	3	.016	5	.956	3
48			min	-7.541	5	-20.665	1	-56.737	4	-.015	2	-.11	1	-.807	2
49		6	max	39.025	1	223.828	3	22.176	1	.014	3	-.006	12	.835	3
50			min	-17.703	5	-144.827	2	-49.171	5	-.015	2	-.108	1	-.737	2
51		7	max	39.025	1	390.911	3	60.324	1	.014	3	-.006	12	.57	3
52			min	-27.864	5	-271.214	2	-46.435	5	-.015	2	-.087	4	-.558	2
53		8	max	39.025	1	557.995	3	98.472	1	.014	3	.003	2	.162	3
54			min	-38.026	5	-397.602	2	-43.699	5	-.015	2	-.111	4	-.27	2
55		9	max	39.025	1	725.079	3	136.62	1	.014	3	.097	1	.142	1
56			min	-48.187	5	-523.989	2	-40.963	5	-.015	2	-.144	5	-.391	3
57		10	max	72.796	4	650.376	2	-6.209	12	.015	2	.266	4	.633	2
58			min	2.908	12	-892.162	3	-174.768	1	-.014	3	.003	12	-1.087	3
59		11	max	62.634	4	523.989	2	-4.411	12	.015	2	.181	4	.142	1
60			min	2.908	12	-725.079	3	-136.62	1	-.014	3	-.003	3	-.391	3
61		12	max	52.473	4	397.602	2	-2.614	12	.015	2	.107	4	.162	3
62			min	2.908	12	-557.995	3	-98.472	1	-.014	3	-.007	3	-.27	2
63		13	max	42.311	4	271.214	2	-.816	12	.015	2	.058	5	.57	3
64			min	2.908	12	-390.911	3	-69.439	4	-.014	3	-.072	1	-.558	2
65		14	max	39.025	1	144.827	2	1.664	3	.015	2	.012	5	.835	3
66			min	2.908	12	-223.828	3	-57.879	4	-.014	3	-.108	1	-.737	2
67		15	max	39.025	1	20.665	1	15.972	1	.015	2	-.004	12	.956	3
68			min	2.908	12	-56.744	3	-49.443	5	-.014	3	-.11	1	-.807	2
69		16	max	39.025	1	110.34	3	54.12	1	.015	2	-.001	12	.932	3
70			min	1.638	15	-107.947	2	-46.707	5	-.014	3	-.093	4	-.769	2
71		17	max	39.025	1	277.424	3	92.269	1	.015	2	.006	3	.766	3
72			min	-7.699	5	-234.335	2	-43.971	5	-.014	3	-.118	4	-.621	2
73		18	max	39.025	1	444.507	3	130.417	1	.015	2	.079	1	.455	3
74			min	-17.861	5	-360.722	2	-41.235	5	-.014	3	-.149	5	-.365	2
75		19	max	39.025	1	611.591	3	168.565	1	.015	2	.207	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-28.022	5	-487.109	2	-38.499	5	-.014	3	-.183	5	0	3
77	M15	1	max	82.561	5	690.092	2	-9.795	12	.016	2	.334	4	0	2
78			min	-41.131	1	-351.569	3	-168.549	1	-.012	3	.017	12	0	3
79		2	max	72.399	5	503.591	2	-7.997	12	.016	2	.235	4	.264	3
80			min	-41.131	1	-261.775	3	-130.401	1	-.012	3	.006	10	-.514	2
81		3	max	62.238	5	317.09	2	-6.199	12	.016	2	.148	5	.451	3
82			min	-41.131	1	-171.98	3	-96.866	4	-.012	3	-.017	1	-.867	2
83		4	max	52.076	5	130.589	2	-4.402	12	.016	2	.085	5	.56	3
84			min	-41.131	1	-82.186	3	-85.306	4	-.012	3	-.08	1	-1.06	2
85		5	max	41.914	5	7.609	3	-.761	10	.016	2	.025	5	.592	3
86			min	-41.131	1	-55.912	2	-73.747	4	-.012	3	-.111	1	-1.092	2
87		6	max	31.753	5	97.403	3	22.192	1	.016	2	-.006	12	.547	3
88			min	-41.131	1	-242.413	2	-66.123	5	-.012	3	-.108	1	-.964	2
89		7	max	21.591	5	187.198	3	60.34	1	.016	2	-.006	12	.425	3
90			min	-41.131	1	-428.914	2	-63.387	5	-.012	3	-.107	4	-.675	2
91		8	max	11.43	5	276.992	3	98.488	1	.016	2	.002	2	.225	3
92			min	-41.131	1	-615.415	2	-60.651	5	-.012	3	-.146	4	-.225	2
93		9	max	1.268	5	366.787	3	136.636	1	.016	2	.097	1	.385	2
94			min	-41.131	1	-801.916	2	-57.915	5	-.012	3	-.193	5	-.052	3
95		10	max	-3.717	12	988.417	2	-6.383	12	.012	3	.331	4	1.156	2
96			min	-41.131	1	-456.581	3	-174.784	1	-.016	2	.004	12	-.407	3
97		11	max	-3.717	12	801.916	2	-4.586	12	.012	3	.231	4	.385	2
98			min	-41.131	1	-366.787	3	-136.636	1	-.016	2	-.002	3	-.052	3
99		12	max	-3.717	12	615.415	2	-2.788	12	.012	3	.142	4	.225	3
100			min	-41.131	1	-276.992	3	-98.488	1	-.016	2	-.007	3	-.225	2
101		13	max	-3.717	12	428.914	2	-.991	12	.012	3	.078	5	.425	3
102			min	-41.131	1	-187.198	3	-86.496	4	-.016	2	-.072	1	-.675	2
103		14	max	-3.717	12	242.413	2	1.385	3	.012	3	.018	5	.547	3
104			min	-48.915	4	-97.403	3	-74.936	4	-.016	2	-.108	1	-.964	2
105		15	max	-3.717	12	55.912	2	15.956	1	.012	3	-.005	12	.592	3
106			min	-59.077	4	-7.609	3	-66.4	5	-.016	2	-.111	1	-1.092	2
107		16	max	-3.717	12	82.186	3	54.104	1	.012	3	-.001	12	.56	3
108			min	-69.238	4	-130.589	2	-63.664	5	-.016	2	-.116	4	-1.06	2
109		17	max	-3.717	12	171.98	3	92.253	1	.012	3	.005	3	.451	3
110			min	-79.4	4	-317.09	2	-60.928	5	-.016	2	-.156	4	-.867	2
111		18	max	-3.717	12	261.775	3	130.401	1	.012	3	.078	1	.264	3
112			min	-89.561	4	-503.591	2	-58.192	5	-.016	2	-.202	5	-.514	2
113		19	max	-3.717	12	351.569	3	168.549	1	.012	3	.207	1	0	2
114			min	-99.723	4	-690.092	2	-55.456	5	-.016	2	-.251	5	0	5
115	M16	1	max	76.996	5	621.99	2	-9.053	12	.008	1	.239	4	0	2
116			min	-69.389	1	-291.627	3	-162.137	1	-.013	3	.013	12	0	3
117		2	max	66.834	5	435.489	2	-7.255	12	.008	1	.162	4	.212	3
118			min	-69.389	1	-201.833	3	-123.989	1	-.013	3	.004	10	-.455	2
119		3	max	56.673	5	248.988	2	-5.458	12	.008	1	.103	5	.348	3
120			min	-69.389	1	-112.039	3	-85.84	1	-.013	3	-.04	1	-.75	2
121		4	max	46.511	5	62.487	2	-3.66	12	.008	1	.06	5	.405	3
122			min	-69.389	1	-22.244	3	-60.313	4	-.013	3	-.097	1	-.884	2
123		5	max	36.349	5	67.55	3	-.198	10	.008	1	.019	5	.386	3
124			min	-69.389	1	-124.014	2	-48.753	4	-.013	3	-.122	1	-.858	2
125		6	max	26.188	5	157.345	3	28.604	1	.008	1	-.006	12	.289	3
126			min	-69.389	1	-310.515	2	-42.967	5	-.013	3	-.114	1	-.671	2
127		7	max	16.026	5	247.139	3	66.752	1	.008	1	-.006	12	.115	3
128			min	-69.389	1	-497.016	2	-40.231	5	-.013	3	-.073	4	-.323	2
129		8	max	5.865	5	336.934	3	104.9	1	.008	1	.004	2	.185	2
130			min	-69.389	1	-683.518	2	-37.495	5	-.013	3	-.09	4	-.137	3
131		9	max	-2.812	15	426.728	3	143.049	1	.008	1	.108	1	.854	2
132			min	-69.389	1	-870.019	2	-34.759	5	-.013	3	-.119	5	-.465	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-5.388	12	1056.52	2	-7.125	12	.013	3	.248	1	1.684	2
134		min	-69.389	1	-516.523	3	-181.197	1	-.008	1	.006	12	-.871	3
135	11	max	-3.741	15	870.019	2	-5.327	12	.013	3	.161	4	.854	2
136		min	-69.389	1	-426.728	3	-143.049	1	-.008	1	0	3	-.465	3
137	12	max	-5.388	12	683.518	2	-3.53	12	.013	3	.089	4	.185	2
138		min	-69.389	1	-336.934	3	-104.9	1	-.008	1	-.005	3	-.137	3
139	13	max	-5.388	12	497.016	2	-1.732	12	.013	3	.044	5	.115	3
140		min	-69.389	1	-247.139	3	-66.752	1	-.008	1	-.073	1	-.323	2
141	14	max	-5.388	12	310.515	2	.231	3	.013	3	.003	5	.289	3
142		min	-69.389	1	-157.345	3	-53.991	4	-.008	1	-.114	1	-.671	2
143	15	max	-5.388	12	124.014	2	9.544	1	.013	3	-.005	12	.386	3
144		min	-69.389	1	-67.55	3	-44.222	5	-.008	1	-.122	1	-.858	2
145	16	max	-5.388	12	22.244	3	47.692	1	.013	3	-.003	12	.405	3
146		min	-73.668	4	-62.487	2	-41.486	5	-.008	1	-.097	1	-.884	2
147	17	max	-5.388	12	112.039	3	85.84	1	.013	3	.002	3	.348	3
148		min	-83.83	4	-248.988	2	-38.75	5	-.008	1	-.118	4	-.75	2
149	18	max	-5.388	12	201.833	3	123.989	1	.013	3	.051	1	.212	3
150		min	-93.992	4	-435.489	2	-36.014	5	-.008	1	-.14	5	-.455	2
151	19	max	-5.388	12	291.627	3	162.137	1	.013	3	.174	1	0	2
152		min	-104.153	4	-621.99	2	-33.278	5	-.008	1	-.17	5	0	5
153	M2	1	max	1055.368	2	2.06	.34	1	0	3	0	3	0	1
154		min	-1432.838	3	.5	15	-25.399	4	0	4	0	2	0	1
155	2	max	1055.898	2	1.989	4	.34	1	0	3	0	1	0	15
156		min	-1432.441	3	.483	15	-25.86	4	0	4	-.009	4	0	4
157	3	max	1056.427	2	1.918	4	.34	1	0	3	0	1	0	15
158		min	-1432.044	3	.466	15	-26.321	4	0	4	-.019	4	-.001	4
159	4	max	1056.956	2	1.847	4	.34	1	0	3	0	1	0	15
160		min	-1431.647	3	.449	15	-26.782	4	0	4	-.028	4	-.002	4
161	5	max	1057.486	2	1.775	4	.34	1	0	3	0	1	0	15
162		min	-1431.25	3	.433	15	-27.243	4	0	4	-.038	4	-.003	4
163	6	max	1058.015	2	1.704	4	.34	1	0	3	0	1	0	15
164		min	-1430.853	3	.416	15	-27.705	4	0	4	-.048	4	-.003	4
165	7	max	1058.544	2	1.633	4	.34	1	0	3	0	1	0	15
166		min	-1430.456	3	.399	15	-28.166	4	0	4	-.058	4	-.004	4
167	8	max	1059.073	2	1.562	4	.34	1	0	3	0	1	-.001	15
168		min	-1430.059	3	.383	15	-28.627	4	0	4	-.068	4	-.005	4
169	9	max	1059.603	2	1.491	4	.34	1	0	3	0	1	-.001	15
170		min	-1429.662	3	.366	15	-29.088	4	0	4	-.078	4	-.005	4
171	10	max	1060.132	2	1.42	4	.34	1	0	3	.001	1	-.001	15
172		min	-1429.265	3	.349	15	-29.55	4	0	4	-.089	4	-.006	4
173	11	max	1060.661	2	1.349	4	.34	1	0	3	.001	1	-.001	15
174		min	-1428.868	3	.328	12	-30.011	4	0	4	-.099	4	-.006	4
175	12	max	1061.191	2	1.278	4	.34	1	0	3	.001	1	-.002	15
176		min	-1428.472	3	.301	12	-30.472	4	0	4	-.11	4	-.007	4
177	13	max	1061.72	2	1.207	4	.34	1	0	3	.001	1	-.002	15
178		min	-1428.075	3	.273	12	-30.933	4	0	4	-.121	4	-.007	4
179	14	max	1062.249	2	1.136	4	.34	1	0	3	.002	1	-.002	15
180		min	-1427.678	3	.245	12	-31.394	4	0	4	-.132	4	-.007	4
181	15	max	1062.778	2	1.065	4	.34	1	0	3	.002	1	-.002	15
182		min	-1427.281	3	.218	12	-31.856	4	0	4	-.144	4	-.008	4
183	16	max	1063.308	2	.994	4	.34	1	0	3	.002	1	-.002	15
184		min	-1426.884	3	.19	12	-32.317	4	0	4	-.155	4	-.008	4
185	17	max	1063.837	2	.923	4	.34	1	0	3	.002	1	-.002	15
186		min	-1426.487	3	.162	12	-32.778	4	0	4	-.167	4	-.009	4
187	18	max	1064.366	2	.852	4	.34	1	0	3	.002	1	-.002	15
188		min	-1426.09	3	.135	12	-33.239	4	0	4	-.179	4	-.009	4
189	19	max	1064.896	2	.795	2	.34	1	0	3	.002	1	-.002	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1425.693	3	.107	12	-33.701	4	0	4	-.191	4	-.009	4
191	M3	1	max	788.434	2	8.903	4	1.468	4	0	12	0	1	.009	4
192			min	-927.01	3	2.104	15	.022	12	0	4	-.02	4	.002	15
193		2	max	788.264	2	8.034	4	2.073	4	0	12	0	1	.005	4
194			min	-927.138	3	1.9	15	.022	12	0	4	-.02	4	0	12
195		3	max	788.093	2	7.165	4	2.678	4	0	12	0	1	.002	2
196			min	-927.266	3	1.696	15	.022	12	0	4	-.019	4	0	3
197		4	max	787.923	2	6.296	4	3.284	4	0	12	0	1	0	2
198			min	-927.394	3	1.492	15	.022	12	0	4	-.017	4	-.003	3
199		5	max	787.752	2	5.427	4	3.889	4	0	12	0	1	0	15
200			min	-927.522	3	1.287	15	.022	12	0	4	-.015	5	-.004	6
201		6	max	787.582	2	4.558	4	4.494	4	0	12	0	1	-.001	15
202			min	-927.649	3	1.083	15	.022	12	0	4	-.014	5	-.007	6
203		7	max	787.412	2	3.689	4	5.099	4	0	12	.001	1	-.002	15
204			min	-927.777	3	.879	15	.022	12	0	4	-.011	5	-.009	6
205		8	max	787.241	2	2.82	4	5.704	4	0	12	.001	1	-.002	15
206			min	-927.905	3	.675	15	.022	12	0	4	-.009	5	-.01	6
207		9	max	787.071	2	1.952	4	6.309	4	0	12	.001	1	-.003	15
208			min	-928.033	3	.47	15	.022	12	0	4	-.006	5	-.011	6
209		10	max	786.901	2	1.083	4	6.914	4	0	12	.001	1	-.003	15
210			min	-928.16	3	.266	15	.022	12	0	4	-.003	5	-.012	6
211		11	max	786.73	2	.309	2	7.519	4	0	12	.002	1	-.003	15
212			min	-928.288	3	-.115	3	.022	12	0	4	0	12	-.012	6
213		12	max	786.56	2	-.142	15	8.124	4	0	12	.004	4	-.003	15
214			min	-928.416	3	-.656	6	.022	12	0	4	0	12	-.012	6
215		13	max	786.39	2	-.347	15	8.729	4	0	12	.008	4	-.003	15
216			min	-928.544	3	-1.525	6	.022	12	0	4	0	12	-.012	6
217		14	max	786.219	2	-.551	15	9.334	4	0	12	.013	4	-.002	15
218			min	-928.671	3	-2.394	6	.022	12	0	4	0	12	-.011	6
219		15	max	786.049	2	-.755	15	9.939	4	0	12	.017	4	-.002	15
220			min	-928.799	3	-3.263	6	.022	12	0	4	0	12	-.009	6
221		16	max	785.879	2	-.959	15	10.544	4	0	12	.022	4	-.002	15
222			min	-928.927	3	-4.132	6	.022	12	0	4	0	12	-.008	6
223		17	max	785.708	2	-1.164	15	11.149	4	0	12	.027	4	-.001	15
224			min	-929.055	3	-5.001	6	.022	12	0	4	0	12	-.006	6
225		18	max	785.538	2	-1.368	15	11.755	4	0	12	.032	4	0	15
226			min	-929.182	3	-5.87	6	.022	12	0	4	0	12	-.003	6
227		19	max	785.368	2	-1.572	15	12.36	4	0	12	.038	4	0	1
228			min	-929.31	3	-6.738	6	.022	12	0	4	0	12	0	1
229	M4	1	max	966.868	1	0	1	-.835	12	0	1	.031	4	0	1
230			min	-80.365	5	0	1	-268.022	4	0	1	0	12	0	1
231		2	max	967.039	1	0	1	-.835	12	0	1	0	1	0	1
232			min	-80.286	5	0	1	-268.17	4	0	1	0	5	0	1
233		3	max	967.209	1	0	1	-.835	12	0	1	0	12	0	1
234			min	-80.206	5	0	1	-268.318	4	0	1	-.031	4	0	1
235		4	max	967.379	1	0	1	-.835	12	0	1	0	12	0	1
236			min	-80.127	5	0	1	-268.465	4	0	1	-.062	4	0	1
237		5	max	967.55	1	0	1	-.835	12	0	1	0	12	0	1
238			min	-80.047	5	0	1	-268.613	4	0	1	-.093	4	0	1
239		6	max	967.72	1	0	1	-.835	12	0	1	0	12	0	1
240			min	-79.968	5	0	1	-268.76	4	0	1	-.123	4	0	1
241		7	max	967.89	1	0	1	-.835	12	0	1	0	12	0	1
242			min	-79.888	5	0	1	-268.908	4	0	1	-.154	4	0	1
243		8	max	968.061	1	0	1	-.835	12	0	1	0	12	0	1
244			min	-79.809	5	0	1	-269.056	4	0	1	-.185	4	0	1
245		9	max	968.231	1	0	1	-.835	12	0	1	0	12	0	1
246			min	-79.729	5	0	1	-269.203	4	0	1	-.216	4	0	1





Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	968.401	1	0	1	-.835	12	0	1	0	12	0	1
248		min	-79.65	5	0	1	-269.351	4	0	1	-.247	4	0	1
249	11	max	968.572	1	0	1	-.835	12	0	1	0	12	0	1
250		min	-79.57	5	0	1	-269.499	4	0	1	-.278	4	0	1
251	12	max	968.742	1	0	1	-.835	12	0	1	0	12	0	1
252		min	-79.491	5	0	1	-269.646	4	0	1	-.309	4	0	1
253	13	max	968.912	1	0	1	-.835	12	0	1	0	12	0	1
254		min	-79.411	5	0	1	-269.794	4	0	1	-.34	4	0	1
255	14	max	969.083	1	0	1	-.835	12	0	1	-.001	12	0	1
256		min	-79.332	5	0	1	-269.941	4	0	1	-.371	4	0	1
257	15	max	969.253	1	0	1	-.835	12	0	1	-.001	12	0	1
258		min	-79.252	5	0	1	-270.089	4	0	1	-.402	4	0	1
259	16	max	969.423	1	0	1	-.835	12	0	1	-.001	12	0	1
260		min	-79.173	5	0	1	-270.237	4	0	1	-.433	4	0	1
261	17	max	969.594	1	0	1	-.835	12	0	1	-.001	12	0	1
262		min	-79.093	5	0	1	-270.384	4	0	1	-.464	4	0	1
263	18	max	969.764	1	0	1	-.835	12	0	1	-.001	12	0	1
264		min	-79.014	5	0	1	-270.532	4	0	1	-.495	4	0	1
265	19	max	969.934	1	0	1	-.835	12	0	1	-.002	12	0	1
266		min	-78.934	5	0	1	-270.68	4	0	1	-.526	4	0	1
267	M6	1	max	3206.584	2	2.275	2	0	1	0	0	4	0	1
268		min	-4474.54	3	.23	12	-25.687	4	0	4	0	1	0	1
269	2	max	3207.114	2	2.219	2	0	1	0	1	0	1	0	12
270		min	-4474.143	3	.203	12	-26.148	4	0	4	-.009	4	0	2
271	3	max	3207.643	2	2.164	2	0	1	0	1	0	1	0	12
272		min	-4473.747	3	.175	12	-26.609	4	0	4	-.019	4	-.002	2
273	4	max	3208.172	2	2.108	2	0	1	0	1	0	1	0	12
274		min	-4473.35	3	.147	12	-27.071	4	0	4	-.028	4	-.002	2
275	5	max	3208.702	2	2.053	2	0	1	0	1	0	1	0	12
276		min	-4472.953	3	.119	12	-27.532	4	0	4	-.038	4	-.003	2
277	6	max	3209.231	2	1.998	2	0	1	0	1	0	1	0	12
278		min	-4472.556	3	.082	3	-27.993	4	0	4	-.048	4	-.004	2
279	7	max	3209.76	2	1.942	2	0	1	0	1	0	1	0	12
280		min	-4472.159	3	.041	3	-28.454	4	0	4	-.058	4	-.005	2
281	8	max	3210.29	2	1.887	2	0	1	0	1	0	1	0	12
282		min	-4471.762	3	0	3	-28.915	4	0	4	-.069	4	-.005	2
283	9	max	3210.819	2	1.832	2	0	1	0	1	0	1	0	12
284		min	-4471.365	3	-.042	3	-29.377	4	0	4	-.079	4	-.006	2
285	10	max	3211.348	2	1.776	2	0	1	0	1	0	1	0	3
286		min	-4470.968	3	-.084	3	-29.838	4	0	4	-.09	4	-.007	2
287	11	max	3211.877	2	1.721	2	0	1	0	1	0	1	0	3
288		min	-4470.571	3	-.126	3	-30.299	4	0	4	-.1	4	-.007	2
289	12	max	3212.407	2	1.666	2	0	1	0	1	0	1	0	3
290		min	-4470.174	3	-.167	3	-30.76	4	0	4	-.111	4	-.008	2
291	13	max	3212.936	2	1.61	2	0	1	0	1	0	1	0	3
292		min	-4469.777	3	-.209	3	-31.222	4	0	4	-.122	4	-.008	2
293	14	max	3213.465	2	1.555	2	0	1	0	1	0	1	0	3
294		min	-4469.38	3	-.25	3	-31.683	4	0	4	-.134	4	-.009	2
295	15	max	3213.995	2	1.5	2	0	1	0	1	0	1	0	3
296		min	-4468.983	3	-.292	3	-32.144	4	0	4	-.145	4	-.009	2
297	16	max	3214.524	2	1.444	2	0	1	0	1	0	1	0	3
298		min	-4468.586	3	-.333	3	-32.605	4	0	4	-.157	4	-.01	2
299	17	max	3215.053	2	1.389	2	0	1	0	1	0	1	0	3
300		min	-4468.189	3	-.375	3	-33.066	4	0	4	-.169	4	-.011	2
301	18	max	3215.582	2	1.334	2	0	1	0	1	0	1	0	3
302		min	-4467.792	3	-.416	3	-33.528	4	0	4	-.181	4	-.011	2
303	19	max	3216.112	2	1.278	2	0	1	0	1	0	1	0	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4467.395	3	-.458	3	-33.989	4	0	4	-.193	4	-.011	2
305	M7	1	max	2639.106	2	8.9	6	1.056	4	0	1	0	1	.011	2
306			min	-2772.7	3	2.09	15	0	1	0	4	-.021	4	0	3
307		2	max	2638.935	2	8.031	6	1.661	4	0	1	0	1	.008	2
308			min	-2772.828	3	1.886	15	0	1	0	4	-.02	4	-.003	3
309		3	max	2638.765	2	7.162	6	2.266	4	0	1	0	1	.005	2
310			min	-2772.956	3	1.682	15	0	1	0	4	-.019	4	-.004	3
311		4	max	2638.595	2	6.293	6	2.871	4	0	1	0	1	.002	2
312			min	-2773.084	3	1.477	15	0	1	0	4	-.018	4	-.006	3
313		5	max	2638.424	2	5.424	6	3.476	4	0	1	0	1	0	2
314			min	-2773.211	3	1.273	15	0	1	0	4	-.016	4	-.007	3
315		6	max	2638.254	2	4.555	6	4.081	4	0	1	0	1	-.002	15
316			min	-2773.339	3	1.069	15	0	1	0	4	-.015	4	-.008	3
317		7	max	2638.084	2	3.686	6	4.686	4	0	1	0	1	-.002	15
318			min	-2773.467	3	.865	15	0	1	0	4	-.013	4	-.009	3
319		8	max	2637.913	2	2.818	6	5.291	4	0	1	0	1	-.002	15
320			min	-2773.595	3	.66	15	0	1	0	4	-.01	4	-.01	4
321		9	max	2637.743	2	2.031	2	5.896	4	0	1	0	1	-.003	15
322			min	-2773.722	3	.324	12	0	1	0	4	-.008	4	-.011	4
323		10	max	2637.573	2	1.354	2	6.501	4	0	1	0	1	-.003	15
324			min	-2773.85	3	-.064	3	0	1	0	4	-.005	5	-.012	4
325		11	max	2637.402	2	.677	2	7.107	4	0	1	0	1	-.003	15
326			min	-2773.978	3	-.572	3	0	1	0	4	-.002	5	-.012	4
327		12	max	2637.232	2	0	2	7.712	4	0	1	.002	4	-.003	15
328			min	-2774.106	3	-1.08	3	0	1	0	4	0	1	-.012	4
329		13	max	2637.062	2	-.361	15	8.317	4	0	1	.006	4	-.003	15
330			min	-2774.233	3	-1.587	3	0	1	0	4	0	1	-.012	4
331		14	max	2636.891	2	-.565	15	8.922	4	0	1	.01	4	-.003	15
332			min	-2774.361	3	-2.396	4	0	1	0	4	0	1	-.011	4
333		15	max	2636.721	2	-.769	15	9.527	4	0	1	.014	4	-.002	15
334			min	-2774.489	3	-3.265	4	0	1	0	4	0	1	-.009	4
335		16	max	2636.551	2	-.974	15	10.132	4	0	1	.019	4	-.002	15
336			min	-2774.617	3	-4.134	4	0	1	0	4	0	1	-.008	4
337		17	max	2636.38	2	-1.178	15	10.737	4	0	1	.024	4	-.001	15
338			min	-2774.744	3	-5.002	4	0	1	0	4	0	1	-.006	4
339		18	max	2636.21	2	-1.382	15	11.342	4	0	1	.029	4	0	15
340			min	-2774.872	3	-5.871	4	0	1	0	4	0	1	-.003	4
341		19	max	2636.039	2	-1.586	15	11.947	4	0	1	.034	4	0	1
342			min	-2775	3	-6.74	4	0	1	0	4	0	1	0	1
343	M8	1	max	2504.728	1	0	1	0	1	0	1	.028	4	0	1
344			min	-161.199	3	0	1	-256.072	4	0	1	0	1	0	1
345		2	max	2504.898	1	0	1	0	1	0	1	0	1	0	1
346			min	-161.071	3	0	1	-256.22	4	0	1	-.002	4	0	1
347		3	max	2505.069	1	0	1	0	1	0	1	0	1	0	1
348			min	-160.943	3	0	1	-256.368	4	0	1	-.031	4	0	1
349		4	max	2505.239	1	0	1	0	1	0	1	0	1	0	1
350			min	-160.815	3	0	1	-256.515	4	0	1	-.061	4	0	1
351		5	max	2505.41	1	0	1	0	1	0	1	0	1	0	1
352			min	-160.688	3	0	1	-256.663	4	0	1	-.09	4	0	1
353		6	max	2505.58	1	0	1	0	1	0	1	0	1	0	1
354			min	-160.56	3	0	1	-256.811	4	0	1	-.12	4	0	1
355		7	max	2505.75	1	0	1	0	1	0	1	0	1	0	1
356			min	-160.432	3	0	1	-256.958	4	0	1	-.149	4	0	1
357		8	max	2505.921	1	0	1	0	1	0	1	0	1	0	1
358			min	-160.304	3	0	1	-257.106	4	0	1	-.179	4	0	1
359		9	max	2506.091	1	0	1	0	1	0	1	0	1	0	1
360			min	-160.177	3	0	1	-257.254	4	0	1	-.208	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2506.261	1	0	1	0	1	0	1	0	1	0	1
362			min	-160.049	3	0	1	-257.401	4	0	1	-.238	4	0	1
363		11	max	2506.432	1	0	1	0	1	0	1	0	1	0	1
364			min	-159.921	3	0	1	-257.549	4	0	1	-.267	4	0	1
365		12	max	2506.602	1	0	1	0	1	0	1	0	1	0	1
366			min	-159.793	3	0	1	-257.696	4	0	1	-.297	4	0	1
367		13	max	2506.772	1	0	1	0	1	0	1	0	1	0	1
368			min	-159.666	3	0	1	-257.844	4	0	1	-.326	4	0	1
369		14	max	2506.943	1	0	1	0	1	0	1	0	1	0	1
370			min	-159.538	3	0	1	-257.992	4	0	1	-.356	4	0	1
371		15	max	2507.113	1	0	1	0	1	0	1	0	1	0	1
372			min	-159.41	3	0	1	-258.139	4	0	1	-.386	4	0	1
373		16	max	2507.283	1	0	1	0	1	0	1	0	1	0	1
374			min	-159.282	3	0	1	-258.287	4	0	1	-.415	4	0	1
375		17	max	2507.454	1	0	1	0	1	0	1	0	1	0	1
376			min	-159.155	3	0	1	-258.435	4	0	1	-.445	4	0	1
377		18	max	2507.624	1	0	1	0	1	0	1	0	1	0	1
378			min	-159.027	3	0	1	-258.582	4	0	1	-.475	4	0	1
379		19	max	2507.794	1	0	1	0	1	0	1	0	1	0	1
380			min	-158.899	3	0	1	-258.73	4	0	1	-.504	4	0	1
381	M10	1	max	1055.368	2	1.99	6	-.029	12	0	1	0	4	0	1
382			min	-1432.838	3	.452	15	-25.61	4	0	5	0	3	0	1
383		2	max	1055.898	2	1.919	6	-.029	12	0	1	0	10	0	15
384			min	-1432.441	3	.436	15	-26.071	4	0	5	-.009	4	0	6
385		3	max	1056.427	2	1.848	6	-.029	12	0	1	0	10	0	15
386			min	-1432.044	3	.419	15	-26.532	4	0	5	-.019	4	-.001	6
387		4	max	1056.956	2	1.777	6	-.029	12	0	1	0	10	0	15
388			min	-1431.647	3	.402	15	-26.993	4	0	5	-.028	4	-.002	6
389		5	max	1057.486	2	1.705	6	-.029	12	0	1	0	10	0	15
390			min	-1431.25	3	.385	15	-27.454	4	0	5	-.038	4	-.003	6
391		6	max	1058.015	2	1.634	6	-.029	12	0	1	0	10	0	15
392			min	-1430.853	3	.369	15	-27.916	4	0	5	-.048	4	-.003	6
393		7	max	1058.544	2	1.563	6	-.029	12	0	1	0	10	0	15
394			min	-1430.456	3	.352	15	-28.377	4	0	5	-.058	4	-.004	6
395		8	max	1059.073	2	1.492	6	-.029	12	0	1	0	12	0	15
396			min	-1430.059	3	.335	15	-28.838	4	0	5	-.068	4	-.004	6
397		9	max	1059.603	2	1.421	6	-.029	12	0	1	0	12	-.001	15
398			min	-1429.662	3	.319	15	-29.299	4	0	5	-.079	4	-.005	6
399		10	max	1060.132	2	1.35	6	-.029	12	0	1	0	12	-.001	15
400			min	-1429.265	3	.302	15	-29.761	4	0	5	-.089	4	-.005	6
401		11	max	1060.661	2	1.279	6	-.029	12	0	1	0	12	-.001	15
402			min	-1428.868	3	.285	15	-30.222	4	0	5	-.1	4	-.006	6
403		12	max	1061.191	2	1.208	6	-.029	12	0	1	0	12	-.001	15
404			min	-1428.472	3	.269	15	-30.683	4	0	5	-.111	4	-.006	6
405		13	max	1061.72	2	1.137	6	-.029	12	0	1	0	12	-.002	15
406			min	-1428.075	3	.252	15	-31.144	4	0	5	-.122	4	-.007	6
407		14	max	1062.249	2	1.072	2	-.029	12	0	1	0	12	-.002	15
408			min	-1427.678	3	.235	15	-31.605	4	0	5	-.133	4	-.007	6
409		15	max	1062.778	2	1.016	2	-.029	12	0	1	0	12	-.002	15
410			min	-1427.281	3	.218	12	-32.067	4	0	5	-.145	4	-.007	6
411		16	max	1063.308	2	.961	2	-.029	12	0	1	0	12	-.002	15
412			min	-1426.884	3	.19	12	-32.528	4	0	5	-.156	4	-.008	6
413		17	max	1063.837	2	.906	2	-.029	12	0	1	0	12	-.002	15
414			min	-1426.487	3	.162	12	-32.989	4	0	5	-.168	4	-.008	6
415		18	max	1064.366	2	.85	2	-.029	12	0	1	0	12	-.002	15
416			min	-1426.09	3	.135	12	-33.45	4	0	5	-.18	4	-.008	6
417		19	max	1064.896	2	.795	2	-.029	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1425.693	3	.107	12	-33.912	4	0	5	-.192	4	-.009	6
419	M11	1	max	788.434	2	8.849	6	1.294	4	0	1	0	12	.009	6
420			min	-927.01	3	2.068	15	-.282	1	0	4	-.021	4	.002	15
421		2	max	788.264	2	7.98	6	1.899	4	0	1	0	12	.005	2
422			min	-927.138	3	1.864	15	-.282	1	0	4	-.02	4	0	12
423		3	max	788.093	2	7.112	6	2.504	4	0	1	0	12	.002	2
424			min	-927.266	3	1.66	15	-.282	1	0	4	-.019	4	0	3
425		4	max	787.923	2	6.243	6	3.109	4	0	1	0	12	0	2
426			min	-927.394	3	1.456	15	-.282	1	0	4	-.017	4	-.003	3
427		5	max	787.752	2	5.374	6	3.714	4	0	1	0	12	-.001	15
428			min	-927.522	3	1.251	15	-.282	1	0	4	-.016	4	-.005	4
429		6	max	787.582	2	4.505	6	4.319	4	0	1	0	12	-.002	15
430			min	-927.649	3	1.047	15	-.282	1	0	4	-.014	4	-.007	4
431		7	max	787.412	2	3.636	6	4.925	4	0	1	0	12	-.002	15
432			min	-927.777	3	.843	15	-.282	1	0	4	-.012	4	-.009	4
433		8	max	787.241	2	2.767	6	5.53	4	0	1	0	12	-.003	15
434			min	-927.905	3	.639	15	-.282	1	0	4	-.009	4	-.01	4
435		9	max	787.071	2	1.898	6	6.135	4	0	1	0	12	-.003	15
436			min	-928.033	3	.434	15	-.282	1	0	4	-.007	4	-.012	4
437		10	max	786.901	2	1.029	6	6.74	4	0	1	0	12	-.003	15
438			min	-928.16	3	.23	15	-.282	1	0	4	-.004	4	-.012	4
439		11	max	786.73	2	.309	2	7.345	4	0	1	0	5	-.003	15
440			min	-928.288	3	-.115	3	-.282	1	0	4	-.002	1	-.012	4
441		12	max	786.56	2	-.178	15	7.95	4	0	1	.004	5	-.003	15
442			min	-928.416	3	-.71	4	-.282	1	0	4	-.002	1	-.012	4
443		13	max	786.39	2	-.383	15	8.555	4	0	1	.007	5	-.003	15
444			min	-928.544	3	-1.578	4	-.282	1	0	4	-.002	1	-.012	4
445		14	max	786.219	2	-.587	15	9.16	4	0	1	.012	5	-.003	15
446			min	-928.671	3	-2.447	4	-.282	1	0	4	-.002	1	-.011	4
447		15	max	786.049	2	-.791	15	9.765	4	0	1	.016	5	-.002	15
448			min	-928.799	3	-3.316	4	-.282	1	0	4	-.002	1	-.01	4
449		16	max	785.879	2	-.995	15	10.37	4	0	1	.021	5	-.002	15
450			min	-928.927	3	-4.185	4	-.282	1	0	4	-.002	1	-.008	4
451		17	max	785.708	2	-1.2	15	10.975	4	0	1	.026	5	-.001	15
452			min	-929.055	3	-5.054	4	-.282	1	0	4	-.002	1	-.006	4
453		18	max	785.538	2	-1.404	15	11.58	4	0	1	.031	5	0	15
454			min	-929.182	3	-5.923	4	-.282	1	0	4	-.002	1	-.003	4
455		19	max	785.368	2	-1.608	15	12.185	4	0	1	.037	5	0	1
456			min	-929.31	3	-6.792	4	-.282	1	0	4	-.003	1	0	1
457	M12	1	max	966.868	1	0	1	11.069	1	0	1	.03	5	0	1
458			min	-21.091	3	0	1	-261.149	4	0	1	-.002	1	0	1
459		2	max	967.039	1	0	1	11.069	1	0	1	0	12	0	1
460			min	-20.963	3	0	1	-261.297	4	0	1	0	1	0	1
461		3	max	967.209	1	0	1	11.069	1	0	1	0	1	0	1
462			min	-20.835	3	0	1	-261.445	4	0	1	-.031	4	0	1
463		4	max	967.379	1	0	1	11.069	1	0	1	.002	1	0	1
464			min	-20.707	3	0	1	-261.592	4	0	1	-.061	4	0	1
465		5	max	967.55	1	0	1	11.069	1	0	1	.003	1	0	1
466			min	-20.58	3	0	1	-261.74	4	0	1	-.091	4	0	1
467		6	max	967.72	1	0	1	11.069	1	0	1	.004	1	0	1
468			min	-20.452	3	0	1	-261.887	4	0	1	-.121	4	0	1
469		7	max	967.89	1	0	1	11.069	1	0	1	.006	1	0	1
470			min	-20.324	3	0	1	-262.035	4	0	1	-.151	4	0	1
471		8	max	968.061	1	0	1	11.069	1	0	1	.007	1	0	1
472			min	-20.196	3	0	1	-262.183	4	0	1	-.181	4	0	1
473		9	max	968.231	1	0	1	11.069	1	0	1	.008	1	0	1
474			min	-20.069	3	0	1	-262.33	4	0	1	-.211	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	968.401	1	0	1	11.069	1	0	1	.009	1	0	1
476			min	-19.941	3	0	1	-262.478	4	0	1	-.241	4	0	1
477		11	max	968.572	1	0	1	11.069	1	0	1	.011	1	0	1
478			min	-19.813	3	0	1	-262.626	4	0	1	-.271	4	0	1
479		12	max	968.742	1	0	1	11.069	1	0	1	.012	1	0	1
480			min	-19.685	3	0	1	-262.773	4	0	1	-.302	4	0	1
481		13	max	968.912	1	0	1	11.069	1	0	1	.013	1	0	1
482			min	-19.558	3	0	1	-262.921	4	0	1	-.332	4	0	1
483		14	max	969.083	1	0	1	11.069	1	0	1	.014	1	0	1
484			min	-19.43	3	0	1	-263.069	4	0	1	-.362	4	0	1
485		15	max	969.253	1	0	1	11.069	1	0	1	.016	1	0	1
486			min	-19.302	3	0	1	-263.216	4	0	1	-.392	4	0	1
487		16	max	969.423	1	0	1	11.069	1	0	1	.017	1	0	1
488			min	-19.174	3	0	1	-263.364	4	0	1	-.422	4	0	1
489		17	max	969.594	1	0	1	11.069	1	0	1	.018	1	0	1
490			min	-19.047	3	0	1	-263.511	4	0	1	-.453	4	0	1
491		18	max	969.764	1	0	1	11.069	1	0	1	.02	1	0	1
492			min	-18.919	3	0	1	-263.659	4	0	1	-.483	4	0	1
493		19	max	969.934	1	0	1	11.069	1	0	1	.021	1	0	1
494			min	-18.791	3	0	1	-263.807	4	0	1	-.513	4	0	1
495	M1	1	max	161.543	1	741.491	3	43.901	5	0	2	.171	1	0	15
496			min	-19.339	5	-414.302	2	-61.169	1	0	3	-.114	5	-.014	2
497		2	max	162.385	1	740.397	3	45.362	5	0	2	.133	1	.243	2
498			min	-18.946	5	-415.761	2	-61.169	1	0	3	-.086	5	-.464	3
499		3	max	599.842	3	540.916	2	22.086	5	0	3	.095	1	.491	2
500			min	-360.862	2	-575.26	3	-60.975	1	0	2	-.058	5	-.909	3
501		4	max	600.474	3	539.457	2	23.546	5	0	3	.057	1	.157	1
502			min	-360.019	2	-576.355	3	-60.975	1	0	2	-.044	5	-.551	3
503		5	max	601.106	3	537.998	2	25.006	5	0	3	.02	1	-.005	15
504			min	-359.177	2	-577.449	3	-60.975	1	0	2	-.029	5	-.193	3
505		6	max	601.737	3	536.538	2	26.466	5	0	3	-.001	12	.165	3
506			min	-358.334	2	-578.543	3	-60.975	1	0	2	-.018	1	-.512	2
507		7	max	602.369	3	535.079	2	27.926	5	0	3	.004	5	.525	3
508			min	-357.492	2	-579.637	3	-60.975	1	0	2	-.056	1	-.844	2
509		8	max	603.001	3	533.62	2	29.386	5	0	3	.022	5	.885	3
510			min	-356.65	2	-580.732	3	-60.975	1	0	2	-.094	1	-1.176	2
511		9	max	618.777	3	50.633	2	58.685	5	0	9	.061	1	1.03	3
512			min	-287.249	2	.437	15	-100.046	1	0	3	-.132	5	-1.343	2
513		10	max	619.409	3	49.174	2	60.145	5	0	9	0	10	1.008	3
514			min	-286.407	2	-.007	5	-100.046	1	0	3	-.096	4	-1.374	2
515		11	max	620.041	3	47.715	2	61.605	5	0	9	-.005	12	.987	3
516			min	-285.564	2	-1.833	4	-100.046	1	0	3	-.073	4	-1.404	2
517		12	max	635.54	3	391.196	3	151.993	5	0	2	.092	1	.865	3
518			min	-216.057	2	-640.705	2	-58.938	1	0	3	-.247	5	-1.246	2
519		13	max	636.172	3	390.102	3	153.453	5	0	2	.056	1	.623	3
520			min	-215.215	2	-642.165	2	-58.938	1	0	3	-.153	5	-.848	2
521		14	max	636.804	3	389.008	3	154.913	5	0	2	.019	1	.381	3
522			min	-214.372	2	-643.624	2	-58.938	1	0	3	-.057	5	-.449	2
523		15	max	637.436	3	387.913	3	156.373	5	0	2	.04	5	.14	3
524			min	-213.53	2	-645.083	2	-58.938	1	0	3	-.018	1	-.07	1
525		16	max	638.068	3	386.819	3	157.834	5	0	2	.137	5	.352	2
526			min	-212.687	2	-646.542	2	-58.938	1	0	3	-.054	1	-.1	3
527		17	max	638.699	3	385.725	3	159.294	5	0	2	.236	5	.753	2
528			min	-211.845	2	-648.001	2	-58.938	1	0	3	-.091	1	-.34	3
529		18	max	32.884	5	624.287	2	-5.389	12	0	5	.224	5	.379	2
530			min	-162.974	1	-290.676	3	-105.635	4	0	2	-.131	1	-.167	3
531		19	max	33.277	5	622.828	2	-5.389	12	0	5	.17	5	.013	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-162.132	1	-291.77	3	-104.175	4	0	2	-.174	1	-.008	1
533	M5	max	363.581	1	2451.741	3	86.261	5	0	1	0	1	.029	2
534		min	13.161	12	-1440.277	2	0	1	0	4	-.231	4	0	15
535		max	364.424	1	2450.647	3	87.721	5	0	1	0	1	.923	2
536		min	13.582	12	-1441.736	2	0	1	0	4	-.178	4	-1.513	3
537		max	1840.244	3	1443.917	2	67.061	4	0	4	0	1	1.787	2
538		min	-1147.73	2	-1682.312	3	0	1	0	1	-.123	4	-2.989	3
539		max	1840.875	3	1442.458	2	68.521	4	0	4	0	1	.891	2
540		min	-1146.888	2	-1683.406	3	0	1	0	1	-.081	4	-1.944	3
541		max	1841.507	3	1440.999	2	69.981	4	0	4	0	1	.043	1
542		min	-1146.046	2	-1684.501	3	0	1	0	1	-.038	4	-.899	3
543		max	1842.139	3	1439.539	2	71.441	4	0	4	.006	4	.147	3
544		min	-1145.203	2	-1685.595	3	0	1	0	1	0	1	-.898	2
545		max	1842.771	3	1438.08	2	72.901	4	0	4	.051	4	1.193	3
546		min	-1144.361	2	-1686.689	3	0	1	0	1	0	1	-1.791	2
547		max	1843.403	3	1436.621	2	74.361	4	0	4	.096	4	2.24	3
548		min	-1143.518	2	-1687.783	3	0	1	0	1	0	1	-2.683	2
549		max	1862.279	3	171.992	2	198.342	4	0	1	0	1	2.581	3
550		min	-992.658	2	.44	15	0	1	0	1	-.206	4	-3.073	2
551		max	1862.911	3	170.532	2	199.802	4	0	1	0	1	2.496	3
552		min	-991.816	2	0	15	0	1	0	1	-.083	4	-3.18	2
553		max	1863.543	3	169.073	2	201.262	4	0	1	.042	4	2.411	3
554		min	-990.973	2	-1.649	6	0	1	0	1	0	1	-3.285	2
555		max	1882.972	3	1097.277	3	213.288	4	0	1	0	1	2.111	3
556		min	-840.326	2	-1781.036	2	0	1	0	4	-.354	4	-2.937	2
557		max	1883.603	3	1096.183	3	214.748	4	0	1	0	1	1.431	3
558		min	-839.484	2	-1782.495	2	0	1	0	4	-.221	4	-1.831	2
559		max	1884.235	3	1095.089	3	216.208	4	0	1	0	1	.751	3
560		min	-838.642	2	-1783.954	2	0	1	0	4	-.087	4	-.724	2
561		max	1884.867	3	1093.994	3	217.668	4	0	1	.047	4	.383	2
562		min	-837.799	2	-1785.413	2	0	1	0	4	0	1	0	15
563		max	1885.499	3	1092.9	3	219.128	4	0	1	.183	4	1.492	2
564		min	-836.957	2	-1786.873	2	0	1	0	4	0	1	-.607	3
565		max	1886.13	3	1091.806	3	220.589	4	0	1	.319	4	2.601	2
566		min	-836.115	2	-1788.332	2	0	1	0	4	0	1	-1.285	3
567		max	-14.67	12	2117.688	2	0	1	0	4	.348	4	1.329	2
568		min	-363.246	1	-1032.437	3	-20.364	5	0	1	0	1	-.668	3
569		max	-14.248	12	2116.229	2	0	1	0	4	.336	4	.016	1
570		min	-362.403	1	-1033.531	3	-18.903	5	0	1	0	1	-.027	3
571	M9	max	161.543	1	741.491	3	67.749	4	0	3	-.015	12	0	15
572		min	9.597	12	-414.302	2	5.418	12	0	4	-.182	4	-.014	2
573		max	162.385	1	740.397	3	69.209	4	0	3	-.012	12	.243	2
574		min	10.019	12	-415.761	2	5.418	12	0	4	-.139	4	-.464	3
575		max	599.842	3	540.916	2	60.975	1	0	2	-.009	12	.491	2
576		min	-360.862	2	-575.26	3	5.388	12	0	3	-.096	4	-.909	3
577		max	600.474	3	539.457	2	60.975	1	0	2	-.005	12	.157	1
578		min	-360.019	2	-576.355	3	5.388	12	0	3	-.067	4	-.551	3
579		max	601.106	3	537.998	2	60.975	1	0	2	-.002	12	-.005	15
580		min	-359.177	2	-577.449	3	5.388	12	0	3	-.036	4	-.193	3
581		max	601.737	3	536.538	2	60.975	1	0	2	.018	1	.165	3
582		min	-358.334	2	-578.543	3	5.388	12	0	3	-.01	5	-.512	2
583		max	602.369	3	535.079	2	60.975	1	0	2	.056	1	.525	3
584		min	-357.492	2	-579.637	3	5.388	12	0	3	.005	12	-.844	2
585		max	603.001	3	533.62	2	60.975	1	0	2	.094	1	.885	3
586		min	-356.65	2	-580.732	3	5.388	12	0	3	.008	12	-1.176	2
587		max	618.777	3	50.633	2	100.046	1	0	3	-.005	12	1.03	3
588		min	-287.249	2	.454	15	8.289	12	0	9	-.157	4	-1.343	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	619.409	3	49.174	2	100.046	1	0	3	.001	1	1.008	3
590		min	-286.407	2	.014	15	8.289	12	0	9	-.096	4	-1.374	2
591	11	max	620.041	3	47.715	2	100.801	4	0	3	.063	1	.987	3
592		min	-285.564	2	-1.712	6	8.289	12	0	9	-.048	5	-1.404	2
593	12	max	635.54	3	391.196	3	176.602	4	0	3	-.007	12	.865	3
594		min	-216.057	2	-640.705	2	4.578	12	0	2	-.286	4	-1.246	2
595	13	max	636.172	3	390.102	3	178.062	4	0	3	-.004	12	.623	3
596		min	-215.215	2	-642.165	2	4.578	12	0	2	-.176	4	-.848	2
597	14	max	636.804	3	389.008	3	179.523	4	0	3	-.002	12	.381	3
598		min	-214.372	2	-643.624	2	4.578	12	0	2	-.065	4	-.449	2
599	15	max	637.436	3	387.913	3	180.983	4	0	3	.047	4	.14	3
600		min	-213.53	2	-645.083	2	4.578	12	0	2	.001	12	-.07	1
601	16	max	638.068	3	386.819	3	182.443	4	0	3	.16	4	.352	2
602		min	-212.687	2	-646.542	2	4.578	12	0	2	.004	12	-.1	3
603	17	max	638.699	3	385.725	3	183.903	4	0	3	.274	4	.753	2
604		min	-211.845	2	-648.001	2	4.578	12	0	2	.007	12	-.34	3
605	18	max	-9.475	12	624.287	2	69.477	1	0	2	.278	4	.379	2
606		min	-162.974	1	-290.676	3	-78.699	5	0	3	.01	12	-.167	3
607	19	max	-9.053	12	622.828	2	69.477	1	0	2	.239	4	.013	3
608		min	-162.132	1	-291.77	3	-77.239	5	0	3	.013	12	-.008	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.209	2	.011	3	1.441e-2	2	NC	1	NC	1
2				min	-.727	4	-.06	3	-.007	2	-4.177e-3	3	NC	1	NC
3		2	max	0	1	.138	2	.019	1	1.55e-2	2	NC	4	NC	2
4			min	-.727	4	.004	15	-.014	5	-3.872e-3	3	1096.525	3	9544.322	1
5		3	max	0	1	.248	3	.044	1	1.658e-2	2	NC	5	NC	2
6			min	-.728	4	.003	15	-.018	5	-3.567e-3	3	603.622	3	4129.548	1
7		4	max	0	1	.335	3	.065	1	1.767e-2	2	NC	5	NC	3
8			min	-.728	4	.002	15	-.014	5	-3.262e-3	3	471.388	3	2816.51	1
9		5	max	0	1	.359	3	.075	1	1.875e-2	2	NC	5	NC	3
10			min	-.728	4	.002	15	-.005	5	-2.957e-3	3	443.848	3	2455.599	1
11		6	max	0	1	.323	3	.071	1	1.984e-2	2	NC	5	NC	5
12			min	-.728	4	.003	15	-.002	10	-2.652e-3	3	485.916	3	2608.443	1
13		7	max	0	1	.238	3	.053	1	2.093e-2	2	NC	4	NC	2
14			min	-.728	4	.004	15	-.005	10	-2.347e-3	3	624.141	3	3460.115	1
15		8	max	0	1	.255	2	.034	3	2.201e-2	2	NC	4	NC	2
16			min	-.728	4	.006	15	-.01	10	-2.042e-3	3	988.874	3	6641.392	1
17		9	max	0	1	.321	2	.034	3	2.31e-2	2	NC	4	NC	1
18			min	-.728	4	.007	15	-.019	2	-1.737e-3	3	1651.362	2	8145.239	3
19		10	max	0	1	.351	2	.034	3	2.419e-2	2	NC	4	NC	1
20			min	-.728	4	-.018	3	-.024	2	-1.432e-3	3	1308.632	2	8186.074	3
21		11	max	0	12	.321	2	.034	3	2.31e-2	2	NC	4	NC	1
22			min	-.728	4	.007	15	-.019	2	-1.737e-3	3	1651.362	2	8145.239	3
23		12	max	0	12	.255	2	.034	3	2.201e-2	2	NC	4	NC	2
24			min	-.728	4	.005	15	-.011	5	-2.042e-3	3	988.874	3	6641.392	1
25		13	max	0	12	.238	3	.053	1	2.093e-2	2	NC	4	NC	2
26			min	-.728	4	.004	15	-.005	10	-2.347e-3	3	624.141	3	3460.115	1
27		14	max	0	12	.323	3	.071	1	1.984e-2	2	NC	5	NC	5
28			min	-.728	4	.002	15	-.002	10	-2.652e-3	3	485.916	3	2608.443	1
29		15	max	0	12	.359	3	.075	1	1.875e-2	2	NC	5	NC	3
30			min	-.728	4	.001	15	0	10	-2.957e-3	3	443.848	3	2455.599	1
31		16	max	0	12	.335	3	.065	1	1.767e-2	2	NC	5	NC	3
32			min	-.728	4	0	15	0	10	-3.262e-3	3	471.388	3	2816.51	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.248	3	.044	1	1.658e-2	2	NC	5	NC	2
34		min	-728	4	.002	15	0	10	-3.567e-3	3	603.622	3	4129.548	1
35	18	max	0	12	.138	2	.023	4	1.55e-2	2	NC	4	NC	2
36		min	-728	4	.003	15	-.003	10	-3.872e-3	3	1096.525	3	7647.549	4
37	19	max	0	12	.209	2	.011	3	1.441e-2	2	NC	1	NC	1
38		min	-728	4	-.06	3	-.007	2	-4.177e-3	3	NC	1	NC	1
39	M14	1	max	0	.434	3	.01	3	7.981e-3	2	NC	1	NC	1
40		min	-.54	4	-.624	2	-.006	2	-6.44e-3	3	NC	1	NC	1
41	2	max	0	1	.655	3	.012	3	9.142e-3	2	NC	5	NC	1
42		min	-.54	4	-.842	2	-.022	5	-7.5e-3	3	840.444	3	7675.083	5
43	3	max	0	1	.851	3	.033	1	1.03e-2	2	NC	5	NC	2
44		min	-.54	4	-1.038	2	-.027	5	-8.56e-3	3	446.501	3	5510.276	1
45	4	max	0	1	1.001	3	.053	1	1.147e-2	2	NC	15	NC	3
46		min	-.54	4	-1.199	2	-.02	5	-9.62e-3	3	323.717	2	3461.99	1
47	5	max	0	1	1.098	3	.064	1	1.263e-2	2	NC	15	NC	3
48		min	-.54	4	-1.315	2	-.005	5	-1.068e-2	3	269.404	2	2882.416	1
49	6	max	0	1	1.139	3	.062	1	1.379e-2	2	NC	15	NC	3
50		min	-.54	4	-1.384	2	-.001	10	-1.174e-2	3	244.967	2	2971.457	1
51	7	max	0	1	1.132	3	.048	1	1.495e-2	2	NC	15	NC	2
52		min	-.54	4	-1.41	2	-.005	10	-1.28e-2	3	236.841	2	3855.625	1
53	8	max	0	1	1.092	3	.04	4	1.611e-2	2	NC	15	NC	2
54		min	-.54	4	-1.404	2	-.009	10	-1.386e-2	3	238.737	2	4502.738	4
55	9	max	0	1	1.043	3	.03	3	1.727e-2	2	NC	15	NC	1
56		min	-.54	4	-1.382	2	-.017	2	-1.492e-2	3	245.536	2	6399.868	4
57	10	max	0	1	1.018	3	.03	3	1.843e-2	2	NC	15	NC	1
58		min	-.54	4	-1.368	2	-.022	2	-1.598e-2	3	250.013	2	9260.255	3
59	11	max	0	12	1.043	3	.03	3	1.727e-2	2	NC	15	NC	1
60		min	-.54	4	-1.382	2	-.022	5	-1.492e-2	3	245.536	2	8349.404	5
61	12	max	0	12	1.092	3	.03	3	1.611e-2	2	NC	15	NC	2
62		min	-.54	4	-1.404	2	-.026	5	-1.386e-2	3	238.737	2	7172.378	5
63	13	max	0	12	1.132	3	.048	1	1.495e-2	2	NC	15	NC	2
64		min	-.54	4	-1.41	2	-.017	5	-1.28e-2	3	236.841	2	3855.625	1
65	14	max	0	12	1.139	3	.062	1	1.379e-2	2	NC	15	NC	3
66		min	-.54	4	-1.384	2	-.002	5	-1.174e-2	3	244.967	2	2971.457	1
67	15	max	0	12	1.098	3	.064	1	1.263e-2	2	NC	15	NC	3
68		min	-.54	4	-1.315	2	0	10	-1.068e-2	3	269.404	2	2882.416	1
69	16	max	0	12	1.001	3	.053	1	1.147e-2	2	NC	15	NC	3
70		min	-.54	4	-1.199	2	0	10	-9.62e-3	3	323.717	2	3461.99	1
71	17	max	0	12	.851	3	.042	4	1.03e-2	2	NC	5	NC	2
72		min	-.54	4	-1.038	2	-.001	10	-8.56e-3	3	446.501	3	4277.389	4
73	18	max	0	12	.655	3	.028	4	9.142e-3	2	NC	5	NC	1
74		min	-.54	4	-.842	2	-.003	10	-7.5e-3	3	840.444	3	6298.136	4
75	19	max	0	12	.434	3	.01	3	7.981e-3	2	NC	1	NC	1
76		min	-.54	4	-.624	2	-.006	2	-6.44e-3	3	NC	1	NC	1
77	M15	1	max	0	.443	3	.009	3	5.527e-3	3	NC	1	NC	1
78		min	-.436	4	-.623	2	-.006	2	-8.317e-3	2	NC	1	NC	1
79	2	max	0	12	.609	3	.012	1	6.427e-3	3	NC	5	NC	1
80		min	-.436	4	-.885	2	-.031	5	-9.536e-3	2	709.316	2	5631.788	5
81	3	max	0	12	.759	3	.034	1	7.328e-3	3	NC	5	NC	2
82		min	-.436	4	-1.118	2	-.039	5	-1.076e-2	2	375.762	2	4560.066	5
83	4	max	0	12	.884	3	.054	1	8.229e-3	3	NC	15	NC	3
84		min	-.436	4	-1.3	2	-.029	5	-1.198e-2	2	274.531	2	3442.273	1
85	5	max	0	12	.976	3	.065	1	9.13e-3	3	NC	15	NC	3
86		min	-.437	4	-1.421	2	-.01	5	-1.319e-2	2	232.95	2	2865.824	1
87	6	max	0	12	1.033	3	.063	1	1.003e-2	3	NC	15	NC	3
88		min	-.437	4	-1.479	2	0	10	-1.441e-2	2	217.312	2	2951.473	1
89	7	max	0	12	1.059	3	.048	1	1.093e-2	3	NC	15	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.437	4	-1.48	2	-.004	10	-1.563e-2	2	216.923	2	3819.203	1
91	8	max	0	12	1.06	3	.049	4	1.183e-2	3	NC	15	NC	2
92		min	-.437	4	-1.443	2	-.008	10	-1.685e-2	2	226.811	2	3721.148	4
93	9	max	0	12	1.048	3	.035	4	1.273e-2	3	NC	15	NC	1
94		min	-.437	4	-1.393	2	-.016	2	-1.807e-2	2	241.486	2	5124.388	4
95	10	max	0	1	1.039	3	.028	3	1.363e-2	3	NC	15	NC	1
96		min	-.437	4	-1.367	2	-.021	2	-1.929e-2	2	250.051	2	NC	1
97	11	max	0	1	1.048	3	.028	3	1.273e-2	3	NC	15	NC	1
98		min	-.437	4	-1.393	2	-.03	5	-1.807e-2	2	241.486	2	6286.802	5
99	12	max	0	1	1.06	3	.028	3	1.183e-2	3	NC	15	NC	2
100		min	-.437	4	-1.443	2	-.035	5	-1.685e-2	2	226.811	2	5360.402	5
101	13	max	0	1	1.059	3	.048	1	1.093e-2	3	NC	15	NC	2
102		min	-.437	4	-1.48	2	-.023	5	-1.563e-2	2	216.923	2	3819.203	1
103	14	max	0	1	1.033	3	.063	1	1.003e-2	3	NC	15	NC	3
104		min	-.436	4	-1.479	2	-.003	5	-1.441e-2	2	217.312	2	2951.473	1
105	15	max	0	1	.976	3	.065	1	9.13e-3	3	NC	15	NC	3
106		min	-.436	4	-1.421	2	0	10	-1.319e-2	2	232.95	2	2865.824	1
107	16	max	0	1	.884	3	.054	1	8.229e-3	3	NC	15	NC	3
108		min	-.436	4	-1.3	2	0	10	-1.198e-2	2	274.531	2	3442.273	1
109	17	max	0	1	.759	3	.054	4	7.328e-3	3	NC	5	NC	2
110		min	-.436	4	-1.118	2	0	10	-1.076e-2	2	375.762	2	3383.6	4
111	18	max	0	1	.609	3	.037	4	6.427e-3	3	NC	5	NC	1
112		min	-.436	4	-.885	2	-.003	10	-9.536e-3	2	709.316	2	4842.101	4
113	19	max	0	1	.443	3	.009	3	5.527e-3	3	NC	1	NC	1
114		min	-.436	4	-.623	2	-.006	2	-8.317e-3	2	NC	1	NC	1
115	M16	1	max	0	.185	2	.008	3	1.049e-2	3	NC	1	NC	1
116		min	-.127	4	-.155	3	-.005	2	-1.208e-2	2	NC	1	NC	1
117	2	max	0	12	.073	1	.019	1	1.151e-2	3	NC	4	NC	2
118		min	-.127	4	-.112	3	-.022	5	-1.266e-2	2	1513.48	2	7762.862	5
119	3	max	0	12	.017	9	.044	1	1.253e-2	3	NC	5	NC	2
120		min	-.127	4	-.081	3	-.029	5	-1.323e-2	2	846.271	2	4141.665	1
121	4	max	0	12	.008	4	.066	1	1.355e-2	3	NC	5	NC	3
122		min	-.127	4	-.088	2	-.023	5	-1.381e-2	2	680.482	2	2811.467	1
123	5	max	0	12	.009	14	.076	1	1.457e-2	3	NC	5	NC	3
124		min	-.127	4	-.091	2	-.011	5	-1.439e-2	2	674.775	2	2439.561	1
125	6	max	0	12	.022	9	.072	1	1.559e-2	3	NC	4	NC	3
126		min	-.127	4	-.118	3	0	10	-1.497e-2	2	814.781	2	2574.302	1
127	7	max	0	12	.074	1	.055	1	1.66e-2	3	NC	3	NC	2
128		min	-.127	4	-.169	3	-.002	10	-1.554e-2	2	1307.567	2	3372.254	1
129	8	max	0	12	.157	1	.032	4	1.762e-2	3	NC	1	NC	2
130		min	-.127	4	-.227	3	-.006	10	-1.612e-2	2	2592.459	3	5630.816	4
131	9	max	0	12	.24	2	.024	3	1.864e-2	3	NC	4	NC	1
132		min	-.127	4	-.277	3	-.014	2	-1.67e-2	2	1534.393	3	8056.906	4
133	10	max	0	1	.281	2	.024	3	1.966e-2	3	NC	5	NC	1
134		min	-.127	4	-.298	3	-.019	2	-1.728e-2	2	1301.309	3	NC	1
135	11	max	0	1	.24	2	.024	3	1.864e-2	3	NC	4	NC	1
136		min	-.127	4	-.277	3	-.016	5	-1.67e-2	2	1534.393	3	NC	1
137	12	max	0	1	.157	1	.029	1	1.762e-2	3	NC	1	NC	2
138		min	-.127	4	-.227	3	-.017	5	-1.612e-2	2	2592.459	3	6240.011	1
139	13	max	0	1	.074	1	.055	1	1.66e-2	3	NC	3	NC	2
140		min	-.127	4	-.169	3	-.008	5	-1.554e-2	2	1307.567	2	3372.254	1
141	14	max	0	1	.022	9	.072	1	1.559e-2	3	NC	4	NC	3
142		min	-.126	4	-.118	3	0	10	-1.497e-2	2	814.781	2	2574.302	1
143	15	max	0	1	.009	9	.076	1	1.457e-2	3	NC	5	NC	3
144		min	-.126	4	-.091	2	.003	10	-1.439e-2	2	674.775	2	2439.561	1
145	16	max	0	1	.007	6	.066	1	1.355e-2	3	NC	5	NC	3
146		min	-.126	4	-.088	2	.003	10	-1.381e-2	2	680.482	2	2811.467	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.017	9	.048	4	1.253e-2	3	NC	5	NC	2
148			min	-.126	4	-.081	3	0	10	-1.323e-2	2	846.271	2	3832.886	4
149		18	max	0	1	.073	1	.031	4	1.151e-2	3	NC	4	NC	2
150			min	-.126	4	-.112	3	-.002	10	-1.266e-2	2	1513.48	2	5832.056	4
151		19	max	0	1	.185	2	.008	3	1.049e-2	3	NC	1	NC	1
152			min	-.126	4	-.155	3	-.005	2	-1.208e-2	2	NC	1	NC	1
153	M2	1	max	.008	2	.011	2	.008	1	2.185e-3	5	NC	1	NC	2
154			min	-.011	3	-.017	3	-.682	4	-1.797e-4	1	6929.631	2	113.545	4
155		2	max	.007	2	.01	2	.007	1	2.213e-3	5	NC	1	NC	1
156			min	-.01	3	-.017	3	-.627	4	-1.708e-4	1	8086.292	2	123.567	4
157		3	max	.007	2	.008	2	.007	1	2.241e-3	5	NC	1	NC	1
158			min	-.009	3	-.016	3	-.572	4	-1.619e-4	1	9681.299	2	135.458	4
159		4	max	.007	2	.006	2	.006	1	2.269e-3	5	NC	1	NC	1
160			min	-.009	3	-.016	3	-.518	4	-1.53e-4	1	NC	1	149.705	4
161		5	max	.006	2	.005	2	.005	1	2.297e-3	5	NC	1	NC	1
162			min	-.008	3	-.015	3	-.464	4	-1.441e-4	1	NC	1	166.968	4
163		6	max	.006	2	.004	2	.005	1	2.326e-3	5	NC	1	NC	1
164			min	-.008	3	-.015	3	-.412	4	-1.352e-4	1	NC	1	188.163	4
165		7	max	.005	2	.002	2	.004	1	2.354e-3	5	NC	1	NC	1
166			min	-.007	3	-.014	3	-.361	4	-1.263e-4	1	NC	1	214.587	4
167		8	max	.005	2	.001	2	.004	1	2.382e-3	5	NC	1	NC	1
168			min	-.007	3	-.013	3	-.312	4	-1.174e-4	1	NC	1	248.13	4
169		9	max	.004	2	0	2	.003	1	2.41e-3	5	NC	1	NC	1
170			min	-.006	3	-.012	3	-.266	4	-1.085e-4	1	NC	1	291.634	4
171		10	max	.004	2	0	2	.003	1	2.44e-3	4	NC	1	NC	1
172			min	-.005	3	-.012	3	-.222	4	-9.961e-5	1	NC	1	349.538	4
173		11	max	.004	2	-.001	15	.002	1	2.471e-3	4	NC	1	NC	1
174			min	-.005	3	-.011	3	-.181	4	-9.072e-5	1	NC	1	429.091	4
175		12	max	.003	2	-.001	15	.002	1	2.502e-3	4	NC	1	NC	1
176			min	-.004	3	-.01	3	-.143	4	-8.183e-5	1	NC	1	542.816	4
177		13	max	.003	2	-.001	15	.001	1	2.533e-3	4	NC	1	NC	1
178			min	-.004	3	-.009	3	-.109	4	-7.293e-5	1	NC	1	713.91	4
179		14	max	.002	2	-.001	15	0	1	2.563e-3	4	NC	1	NC	1
180			min	-.003	3	-.007	3	-.078	4	-6.404e-5	1	NC	1	989.459	4
181		15	max	.002	2	-.001	15	0	1	2.594e-3	4	NC	1	NC	1
182			min	-.002	3	-.006	3	-.052	4	-5.515e-5	1	NC	1	1477.683	4
183		16	max	.001	2	0	15	0	1	2.625e-3	4	NC	1	NC	1
184			min	-.002	3	-.005	3	-.031	4	-4.625e-5	1	NC	1	2476.51	4
185		17	max	0	2	0	15	0	1	2.656e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	3	-.015	4	-3.736e-5	1	NC	1	5083.584	4
187		18	max	0	2	0	15	0	1	2.687e-3	4	NC	1	NC	1
188			min	0	3	-.002	6	-.005	4	-2.847e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.718e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-1.957e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	3.906e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-5.886e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.015	4	4.036e-5	4	NC	1	NC	1
194			min	0	2	-.003	6	0	1	1.909e-6	12	NC	1	NC	1
195		3	max	.001	3	-.001	15	.028	4	6.693e-4	4	NC	1	NC	1
196			min	0	2	-.006	6	0	1	3.414e-6	12	NC	1	7159.399	14
197		4	max	.002	3	-.002	15	.04	4	1.298e-3	4	NC	1	NC	1
198			min	-.001	2	-.009	6	0	1	4.92e-6	12	NC	1	4959.508	14
199		5	max	.002	3	-.003	15	.052	4	1.927e-3	4	NC	1	NC	1
200			min	-.002	2	-.012	6	0	1	6.426e-6	12	8536.588	6	3862.696	14
201		6	max	.003	3	-.003	15	.062	4	2.556e-3	4	NC	5	NC	1
202			min	-.002	2	-.015	6	0	1	7.932e-6	12	6916.436	6	3205.629	14
203		7	max	.003	3	-.004	15	.072	4	3.185e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.003	2	-.017	6	0	1	9.438e-6	12	5940.713	6	2766.857	14
205		8	max	.004	3	-.004	15	.081	4	3.814e-3	4	NC	5	NC	1
206			min	-.003	2	-.019	6	0	3	1.094e-5	12	5338.92	6	2451.166	14
207		9	max	.004	3	-.004	15	.09	4	4.443e-3	4	NC	5	NC	1
208			min	-.003	2	-.02	6	0	3	1.245e-5	12	4983.736	6	2210.679	14
209		10	max	.005	3	-.005	15	.098	4	5.072e-3	4	NC	5	NC	1
210			min	-.004	2	-.021	6	0	12	1.395e-5	12	4813.43	6	2018.586	14
211		11	max	.005	3	-.005	15	.107	4	5.701e-3	4	NC	5	NC	1
212			min	-.004	2	-.021	6	0	12	1.546e-5	12	4802.959	6	1858.659	14
213		12	max	.006	3	-.004	15	.115	4	6.33e-3	4	NC	5	NC	1
214			min	-.005	2	-.02	6	0	12	1.697e-5	12	4954.369	6	1720.514	14
215		13	max	.006	3	-.004	15	.123	4	6.959e-3	4	NC	5	NC	1
216			min	-.005	2	-.019	6	0	12	1.847e-5	12	5298.82	6	1597.273	14
217		14	max	.007	3	-.004	15	.132	4	7.588e-3	4	NC	5	NC	1
218			min	-.006	2	-.017	6	0	12	1.998e-5	12	5912.674	6	1484.312	14
219		15	max	.007	3	-.003	15	.142	4	8.216e-3	4	NC	3	NC	1
220			min	-.006	2	-.015	6	0	12	2.148e-5	12	6964.953	6	1378.565	14
221		16	max	.008	3	-.002	15	.153	4	8.845e-3	4	NC	1	NC	1
222			min	-.006	2	-.011	6	0	12	2.299e-5	12	8864.841	6	1278.09	14
223		17	max	.008	3	-.001	15	.164	4	9.474e-3	4	NC	1	NC	1
224			min	-.007	2	-.008	6	0	12	2.45e-5	12	NC	1	1181.777	14
225		18	max	.009	3	0	15	.178	4	1.01e-2	4	NC	1	NC	1
226			min	-.007	2	-.005	3	0	12	2.6e-5	12	NC	1	1089.133	14
227		19	max	.009	3	0	5	.193	4	1.073e-2	4	NC	1	NC	1
228			min	-.008	2	-.002	3	0	12	2.751e-5	12	NC	1	1000.093	14
229	M4	1	max	.002	1	.007	2	0	12	1.771e-4	4	NC	1	NC	3
230			min	0	5	-.009	3	-.193	4	1.121e-5	12	NC	1	128.484	4
231		2	max	.002	1	.007	2	0	12	1.771e-4	4	NC	1	NC	3
232			min	0	5	-.009	3	-.178	4	1.121e-5	12	NC	1	139.654	4
233		3	max	.002	1	.007	2	0	12	1.771e-4	4	NC	1	NC	2
234			min	0	5	-.008	3	-.162	4	1.121e-5	12	NC	1	152.95	4
235		4	max	.002	1	.006	2	0	12	1.771e-4	4	NC	1	NC	2
236			min	0	5	-.008	3	-.147	4	1.121e-5	12	NC	1	168.925	4
237		5	max	.002	1	.006	2	0	12	1.771e-4	4	NC	1	NC	2
238			min	0	5	-.007	3	-.132	4	1.121e-5	12	NC	1	188.328	4
239		6	max	.002	1	.005	2	0	12	1.771e-4	4	NC	1	NC	2
240			min	0	5	-.007	3	-.117	4	1.121e-5	12	NC	1	212.199	4
241		7	max	.002	1	.005	2	0	12	1.771e-4	4	NC	1	NC	2
242			min	0	5	-.006	3	-.102	4	1.121e-5	12	NC	1	242.015	4
243		8	max	.001	1	.005	2	0	12	1.771e-4	4	NC	1	NC	2
244			min	0	5	-.006	3	-.089	4	1.121e-5	12	NC	1	279.928	4
245		9	max	.001	1	.004	2	0	12	1.771e-4	4	NC	1	NC	2
246			min	0	5	-.005	3	-.075	4	1.121e-5	12	NC	1	329.178	4
247		10	max	.001	1	.004	2	0	12	1.771e-4	4	NC	1	NC	1
248			min	0	5	-.005	3	-.063	4	1.121e-5	12	NC	1	394.832	4
249		11	max	.001	1	.003	2	0	12	1.771e-4	4	NC	1	NC	1
250			min	0	5	-.004	3	-.051	4	1.121e-5	12	NC	1	485.174	4
251		12	max	0	1	.003	2	0	12	1.771e-4	4	NC	1	NC	1
252			min	0	5	-.004	3	-.04	4	1.121e-5	12	NC	1	614.537	4
253		13	max	0	1	.002	2	0	12	1.771e-4	4	NC	1	NC	1
254			min	0	5	-.003	3	-.031	4	1.121e-5	12	NC	1	809.516	4
255		14	max	0	1	.002	2	0	12	1.771e-4	4	NC	1	NC	1
256			min	0	5	-.003	3	-.022	4	1.121e-5	12	NC	1	1124.211	4
257		15	max	0	1	.002	2	0	12	1.771e-4	4	NC	1	NC	1
258			min	0	5	-.002	3	-.015	4	1.121e-5	12	NC	1	1683.307	4
259		16	max	0	1	.001	2	0	12	1.771e-4	4	NC	1	NC	1
260			min	0	5	-.002	3	-.009	4	1.121e-5	12	NC	1	2831.394	4



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	12	1.771e-4	4	NC	1	NC	1
262		min	0	5	-0.001	3	-0.004	4	1.121e-5	12	NC	1	5845.674	4
263	18	max	0	1	0	2	0	12	1.771e-4	4	NC	1	NC	1
264		min	0	5	0	3	-0.001	4	1.121e-5	12	NC	1	NC	1
265	19	max	0	1	0	1	0	1	1.771e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	1.121e-5	12	NC	1	NC	1
267	M6	1	max	.024	.037	2	0	1	2.287e-3	4	NC	3	NC	1
268		min	-.033	3	-.052	3	-.689	4	0	1	2076.654	2	112.383	4
269	2	max	.023	2	.034	2	0	1	2.314e-3	4	NC	3	NC	1
270		min	-.031	3	-.049	3	-.634	4	0	1	2282.02	2	122.303	4
271	3	max	.021	2	.031	2	0	1	2.34e-3	4	NC	3	NC	1
272		min	-.03	3	-.047	3	-.578	4	0	1	2530.275	2	134.073	4
273	4	max	.02	2	.027	2	0	1	2.367e-3	4	NC	3	NC	1
274		min	-.028	3	-.044	3	-.523	4	0	1	2833.711	2	148.174	4
275	5	max	.019	2	.024	2	0	1	2.394e-3	4	NC	3	NC	1
276		min	-.026	3	-.041	3	-.469	4	0	1	3209.512	2	165.261	4
277	6	max	.017	2	.021	2	0	1	2.42e-3	4	NC	3	NC	1
278		min	-.024	3	-.038	3	-.416	4	0	1	3682.296	2	186.24	4
279	7	max	.016	2	.018	2	0	1	2.447e-3	4	NC	3	NC	1
280		min	-.022	3	-.035	3	-.365	4	0	1	4288.351	2	212.393	4
281	8	max	.015	2	.015	2	0	1	2.473e-3	4	NC	1	NC	1
282		min	-.02	3	-.032	3	-.316	4	0	1	5083.004	2	245.593	4
283	9	max	.013	2	.013	2	0	1	2.5e-3	4	NC	1	NC	1
284		min	-.019	3	-.029	3	-.268	4	0	1	6154.091	2	288.651	4
285	10	max	.012	2	.01	2	0	1	2.527e-3	4	NC	1	NC	1
286		min	-.017	3	-.026	3	-.224	4	0	1	7648.109	2	345.96	4
287	11	max	.011	2	.008	2	0	1	2.553e-3	4	NC	1	NC	1
288		min	-.015	3	-.024	3	-.182	4	0	1	9824.948	2	424.692	4
289	12	max	.009	2	.006	2	0	1	2.58e-3	4	NC	1	NC	1
290		min	-.013	3	-.021	3	-.144	4	0	1	NC	1	537.238	4
291	13	max	.008	2	.004	2	0	1	2.606e-3	4	NC	1	NC	1
292		min	-.011	3	-.018	3	-.11	4	0	1	NC	1	706.546	4
293	14	max	.007	2	.003	2	0	1	2.633e-3	4	NC	1	NC	1
294		min	-.009	3	-.015	3	-.079	4	0	1	NC	1	979.194	4
295	15	max	.005	2	.001	2	0	1	2.66e-3	4	NC	1	NC	1
296		min	-.007	3	-.012	3	-.053	4	0	1	NC	1	1462.21	4
297	16	max	.004	2	0	2	0	1	2.686e-3	4	NC	1	NC	1
298		min	-.006	3	-.009	3	-.032	4	0	1	NC	1	2450.169	4
299	17	max	.003	2	0	2	0	1	2.713e-3	4	NC	1	NC	1
300		min	-.004	3	-.006	3	-.015	4	0	1	NC	1	5027.91	4
301	18	max	.001	2	0	2	0	1	2.739e-3	4	NC	1	NC	1
302		min	-.002	3	-.003	3	-.005	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	2.766e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	0	1	0	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-5.996e-4	4	NC	1	NC	1
307	2	max	.002	3	0	15	.015	4	9.955e-6	4	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	-.001	15	.028	4	6.195e-4	4	NC	1	NC	1
310		min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.005	3	-.002	15	.041	4	1.229e-3	4	NC	1	NC	1
312		min	-.004	2	-.011	3	0	1	0	1	NC	1	9984.883	4
313	5	max	.006	3	-.003	15	.053	4	1.839e-3	4	NC	1	NC	1
314		min	-.006	2	-.014	3	0	1	0	1	8136.622	3	8864.422	4
315	6	max	.008	3	-.004	15	.063	4	2.448e-3	4	NC	1	NC	1
316		min	-.007	2	-.017	3	0	1	0	1	6862.833	3	8594.001	4
317	7	max	.009	3	-.004	15	.073	4	3.058e-3	4	NC	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.009	2	-.019	3	0	1	0	1	5934.696	4	8945.345	4
319	8	max	.011	3	-.005	15	.082	4	3.668e-3	4	NC	2	NC	1
320		min	-.01	2	-.021	3	0	1	0	1	5333.895	4	9968.897	5
321	9	max	.012	3	-.005	15	.091	4	4.277e-3	4	NC	5	NC	1
322		min	-.011	2	-.022	3	0	1	0	1	4979.344	4	NC	1
323	10	max	.014	3	-.005	15	.099	4	4.887e-3	4	NC	5	NC	1
324		min	-.013	2	-.022	3	0	1	0	1	4809.43	4	NC	1
325	11	max	.015	3	-.005	15	.107	4	5.496e-3	4	NC	5	NC	1
326		min	-.014	2	-.022	3	0	1	0	1	4799.171	4	NC	1
327	12	max	.017	3	-.005	15	.115	4	6.106e-3	4	NC	5	NC	1
328		min	-.016	2	-.022	3	0	1	0	1	4950.639	4	NC	1
329	13	max	.018	3	-.005	15	.122	4	6.715e-3	4	NC	5	NC	1
330		min	-.017	2	-.021	3	0	1	0	1	5294.989	4	NC	1
331	14	max	.02	3	-.004	15	.131	4	7.325e-3	4	NC	2	NC	1
332		min	-.019	2	-.019	3	0	1	0	1	5908.547	4	NC	1
333	15	max	.021	3	-.004	15	.139	4	7.935e-3	4	NC	1	NC	1
334		min	-.02	2	-.017	3	0	1	0	1	6960.234	4	NC	1
335	16	max	.023	3	-.003	15	.149	4	8.544e-3	4	NC	1	NC	1
336		min	-.021	2	-.015	3	0	1	0	1	8858.977	4	NC	1
337	17	max	.024	3	-.002	15	.16	4	9.154e-3	4	NC	1	NC	1
338		min	-.023	2	-.012	3	0	1	0	1	NC	1	NC	1
339	18	max	.026	3	-.001	15	.172	4	9.763e-3	4	NC	1	NC	1
340		min	-.024	2	-.009	3	0	1	0	1	NC	1	NC	1
341	19	max	.027	3	0	10	.185	4	1.037e-2	4	NC	1	NC	1
342		min	-.026	2	-.006	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	1	.025	2	0	1	1	NC	1	NC	1
344		min	0	3	-.028	3	-.185	4	-3.079e-5	5	NC	1	133.807	4
345	2	max	.006	1	.024	2	0	1	0	1	NC	1	NC	1
346		min	0	3	-.026	3	-.171	4	-3.079e-5	5	NC	1	145.46	4
347	3	max	.005	1	.022	2	0	1	0	1	NC	1	NC	1
348		min	0	3	-.025	3	-.156	4	-3.079e-5	5	NC	1	159.33	4
349	4	max	.005	1	.021	2	0	1	0	1	NC	1	NC	1
350		min	0	3	-.023	3	-.141	4	-3.079e-5	5	NC	1	175.993	4
351	5	max	.005	1	.02	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.022	3	-.126	4	-3.079e-5	5	NC	1	196.231	4
353	6	max	.004	1	.018	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.02	3	-.112	4	-3.079e-5	5	NC	1	221.129	4
355	7	max	.004	1	.017	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.019	3	-.098	4	-3.079e-5	5	NC	1	252.226	4
357	8	max	.004	1	.015	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.017	3	-.085	4	-3.079e-5	5	NC	1	291.768	4
359	9	max	.003	1	.014	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.015	3	-.072	4	-3.079e-5	5	NC	1	343.135	4
361	10	max	.003	1	.013	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.014	3	-.06	4	-3.079e-5	5	NC	1	411.61	4
363	11	max	.003	1	.011	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.012	3	-.049	4	-3.079e-5	5	NC	1	505.837	4
365	12	max	.002	1	.01	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.011	3	-.039	4	-3.079e-5	5	NC	1	640.764	4
367	13	max	.002	1	.008	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.009	3	-.029	4	-3.079e-5	5	NC	1	844.134	4
369	14	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.008	3	-.021	4	-3.079e-5	5	NC	1	1172.383	4
371	15	max	.001	1	.006	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.006	3	-.014	4	-3.079e-5	5	NC	1	1755.582	4
373	16	max	0	1	.004	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.005	3	-.008	4	-3.079e-5	5	NC	1	2953.223	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	-.004	4	-3.079e-5	5	NC	1	6097.818	4
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.002	3	-.001	4	-3.079e-5	5	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-3.079e-5	5	NC	1	NC	1
381	M10	1	max	.008	2	.011	2	0	12	2.274e-3	4	NC	1	NC	2
382			min	-.011	3	-.017	3	-.687	4	1.733e-5	12	6929.631	2	112.725	4
383		2	max	.007	2	.01	2	0	12	2.3e-3	4	NC	1	NC	1
384			min	-.01	3	-.017	3	-.632	4	1.648e-5	12	8086.292	2	122.676	4
385		3	max	.007	2	.008	2	0	12	2.325e-3	4	NC	1	NC	1
386			min	-.009	3	-.016	3	-.576	4	1.563e-5	12	9681.299	2	134.483	4
387		4	max	.007	2	.006	2	0	12	2.35e-3	4	NC	1	NC	1
388			min	-.009	3	-.016	3	-.521	4	1.477e-5	12	NC	1	148.629	4
389		5	max	.006	2	.005	2	0	12	2.375e-3	4	NC	1	NC	1
390			min	-.008	3	-.015	3	-.467	4	1.392e-5	12	NC	1	165.77	4
391		6	max	.006	2	.004	2	0	12	2.401e-3	4	NC	1	NC	1
392			min	-.008	3	-.015	3	-.415	4	1.307e-5	12	NC	1	186.816	4
393		7	max	.005	2	.002	2	0	12	2.426e-3	4	NC	1	NC	1
394			min	-.007	3	-.014	3	-.364	4	1.221e-5	12	NC	1	213.054	4
395		8	max	.005	2	.001	2	0	12	2.451e-3	4	NC	1	NC	1
396			min	-.007	3	-.013	3	-.315	4	1.136e-5	12	NC	1	246.362	4
397		9	max	.004	2	0	2	0	12	2.477e-3	4	NC	1	NC	1
398			min	-.006	3	-.012	3	-.268	4	1.05e-5	10	NC	1	289.563	4
399		10	max	.004	2	0	2	0	12	2.502e-3	4	NC	1	NC	1
400			min	-.005	3	-.012	3	-.223	4	9.641e-6	10	NC	1	347.064	4
401		11	max	.004	2	-.002	2	0	12	2.527e-3	4	NC	1	NC	1
402			min	-.005	3	-.011	3	-.182	4	8.78e-6	10	NC	1	426.066	4
403		12	max	.003	2	-.002	2	0	12	2.552e-3	4	NC	1	NC	1
404			min	-.004	3	-.01	3	-.144	4	7.918e-6	10	NC	1	539.005	4
405		13	max	.003	2	-.002	15	0	12	2.578e-3	4	NC	1	NC	1
406			min	-.004	3	-.009	3	-.109	4	7.057e-6	10	NC	1	708.924	4
407		14	max	.002	2	-.002	15	0	12	2.603e-3	4	NC	1	NC	1
408			min	-.003	3	-.007	3	-.079	4	6.196e-6	10	NC	1	982.593	4
409		15	max	.002	2	-.002	15	0	12	2.628e-3	4	NC	1	NC	1
410			min	-.002	3	-.006	4	-.053	4	5.334e-6	10	NC	1	1467.514	4
411		16	max	.001	2	-.001	15	0	12	2.654e-3	4	NC	1	NC	1
412			min	-.002	3	-.005	4	-.032	4	4.473e-6	10	NC	1	2459.671	4
413		17	max	0	2	0	15	0	12	2.679e-3	4	NC	1	NC	1
414			min	-.001	3	-.004	4	-.015	4	3.611e-6	10	NC	1	5049.702	4
415		18	max	0	2	0	15	0	12	2.704e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.005	4	2.75e-6	10	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.729e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	1.889e-6	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-3.734e-7	10	NC	1	NC	1
420			min	0	1	0	1	0	1	-5.909e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.015	4	2.929e-5	5	NC	1	NC	1
422			min	0	2	-.003	4	0	10	-2.442e-5	1	NC	1	NC	1
423		3	max	.001	3	-.002	15	.028	4	6.421e-4	5	NC	1	NC	1
424			min	0	2	-.006	4	0	10	-4.493e-5	1	NC	1	NC	1
425		4	max	.002	3	-.002	15	.041	4	1.258e-3	4	NC	1	NC	1
426			min	-.001	2	-.009	4	0	10	-6.544e-5	1	NC	1	NC	1
427		5	max	.002	3	-.003	15	.052	4	1.874e-3	4	NC	1	NC	1
428			min	-.002	2	-.013	4	0	10	-8.595e-5	1	8242.903	4	NC	1
429		6	max	.003	3	-.004	15	.062	4	2.49e-3	4	NC	5	NC	1
430			min	-.002	2	-.016	4	0	10	-1.065e-4	1	6698.993	4	NC	1
431		7	max	.003	3	-.004	15	.072	4	3.106e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.003	2	-.018	4	0	10	-1.27e-4	1	5768.32	4	NC	1
433		8	max	.004	3	-.005	15	.081	4	3.722e-3	4	NC	5	NC	1
434			min	-.003	2	-.02	4	0	10	-1.475e-4	1	5194.682	4	NC	1
435		9	max	.004	3	-.005	15	.09	4	4.338e-3	4	NC	5	NC	1
436			min	-.003	2	-.022	4	0	1	-1.68e-4	1	4857.441	4	NC	1
437		10	max	.005	3	-.006	15	.098	4	4.955e-3	4	NC	5	NC	1
438			min	-.004	2	-.022	4	0	1	-1.885e-4	1	4698.24	4	NC	1
439		11	max	.005	3	-.006	15	.106	4	5.571e-3	4	NC	5	NC	1
440			min	-.004	2	-.022	4	0	1	-2.09e-4	1	4693.747	4	NC	1
441		12	max	.006	3	-.005	15	.114	4	6.187e-3	4	NC	5	NC	1
442			min	-.005	2	-.022	4	-.001	1	-2.295e-4	1	4846.712	4	NC	1
443		13	max	.006	3	-.005	15	.122	4	6.803e-3	4	NC	5	NC	1
444			min	-.005	2	-.021	4	-.002	1	-2.5e-4	1	5188.179	4	NC	1
445		14	max	.007	3	-.005	15	.131	4	7.419e-3	4	NC	5	NC	1
446			min	-.006	2	-.019	4	-.002	1	-2.705e-4	1	5793.403	4	NC	1
447		15	max	.007	3	-.004	15	.14	4	8.036e-3	4	NC	3	NC	1
448			min	-.006	2	-.016	4	-.003	1	-2.911e-4	1	6828.485	4	NC	1
449		16	max	.008	3	-.003	15	.15	4	8.652e-3	4	NC	1	NC	1
450			min	-.006	2	-.013	4	-.004	1	-3.116e-4	1	8695.189	4	NC	1
451		17	max	.008	3	-.002	15	.161	4	9.268e-3	4	NC	1	NC	1
452			min	-.007	2	-.009	4	-.005	1	-3.321e-4	1	NC	1	NC	1
453		18	max	.009	3	-.002	15	.174	4	9.884e-3	4	NC	1	NC	1
454			min	-.007	2	-.006	4	-.006	1	-3.526e-4	1	NC	1	NC	1
455		19	max	.009	3	0	10	.188	4	1.05e-2	4	NC	1	NC	1
456			min	-.008	2	-.002	3	-.007	1	-3.731e-4	1	NC	1	NC	1
457	M12	1	max	.002	1	.007	2	.007	1	1.197e-4	5	NC	1	NC	3
458			min	0	3	-.009	3	-.188	4	-1.402e-4	1	NC	1	131.65	4
459		2	max	.002	1	.007	2	.007	1	1.197e-4	5	NC	1	NC	3
460			min	0	3	-.009	3	-.173	4	-1.402e-4	1	NC	1	143.102	4
461		3	max	.002	1	.007	2	.006	1	1.197e-4	5	NC	1	NC	2
462			min	0	3	-.008	3	-.158	4	-1.402e-4	1	NC	1	156.733	4
463		4	max	.002	1	.006	2	.006	1	1.197e-4	5	NC	1	NC	2
464			min	0	3	-.008	3	-.143	4	-1.402e-4	1	NC	1	173.11	4
465		5	max	.002	1	.006	2	.005	1	1.197e-4	5	NC	1	NC	2
466			min	0	3	-.007	3	-.129	4	-1.402e-4	1	NC	1	193	4
467		6	max	.002	1	.005	2	.005	1	1.197e-4	5	NC	1	NC	2
468			min	0	3	-.007	3	-.114	4	-1.402e-4	1	NC	1	217.471	4
469		7	max	.002	1	.005	2	.004	1	1.197e-4	5	NC	1	NC	2
470			min	0	3	-.006	3	-.1	4	-1.402e-4	1	NC	1	248.036	4
471		8	max	.001	1	.005	2	.003	1	1.197e-4	5	NC	1	NC	2
472			min	0	3	-.006	3	-.086	4	-1.402e-4	1	NC	1	286.901	4
473		9	max	.001	1	.004	2	.003	1	1.197e-4	5	NC	1	NC	2
474			min	0	3	-.005	3	-.074	4	-1.402e-4	1	NC	1	337.389	4
475		10	max	.001	1	.004	2	.002	1	1.197e-4	5	NC	1	NC	1
476			min	0	3	-.005	3	-.061	4	-1.402e-4	1	NC	1	404.691	4
477		11	max	.001	1	.003	2	.002	1	1.197e-4	5	NC	1	NC	1
478			min	0	3	-.004	3	-.05	4	-1.402e-4	1	NC	1	497.303	4
479		12	max	0	1	.003	2	.002	1	1.197e-4	5	NC	1	NC	1
480			min	0	3	-.004	3	-.039	4	-1.402e-4	1	NC	1	629.917	4
481		13	max	0	1	.002	2	.001	1	1.197e-4	5	NC	1	NC	1
482			min	0	3	-.003	3	-.03	4	-1.402e-4	1	NC	1	829.796	4
483		14	max	0	1	.002	2	0	1	1.197e-4	5	NC	1	NC	1
484			min	0	3	-.003	3	-.022	4	-1.402e-4	1	NC	1	1152.404	4
485		15	max	0	1	.002	2	0	1	1.197e-4	5	NC	1	NC	1
486			min	0	3	-.002	3	-.014	4	-1.402e-4	1	NC	1	1725.565	4
487		16	max	0	1	.001	2	0	1	1.197e-4	5	NC	1	NC	1
488			min	0	3	-.002	3	-.009	4	-1.402e-4	1	NC	1	2902.551	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	1.197e-4	5	NC	1	NC	1
490			min	0	3	-.001	3	-.004	4	-1.402e-4	1	NC	1	5992.77	4
491		18	max	0	1	0	2	0	1	1.197e-4	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-1.402e-4	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	1.197e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-1.402e-4	1	NC	1	NC	1
495	M1	1	max	.011	3	.209	2	.728	4	7.168e-3	2	NC	1	NC	1
496			min	-.007	2	-.06	3	0	12	-1.703e-2	3	NC	1	NC	1
497		2	max	.011	3	.101	2	.706	4	6.975e-3	4	NC	5	NC	1
498			min	-.007	2	-.028	3	-.006	1	-8.454e-3	3	1258.061	2	NC	1
499		3	max	.011	3	.018	3	.682	4	1.26e-2	4	NC	5	NC	1
500			min	-.007	2	-.014	2	-.008	1	-1.508e-4	1	609.733	2	5868.621	5
501		4	max	.011	3	.086	3	.657	4	1.084e-2	4	NC	15	NC	1
502			min	-.007	2	-.14	2	-.007	1	-4.141e-3	3	388.444	2	4293.568	5
503		5	max	.011	3	.17	3	.631	4	9.074e-3	4	NC	15	NC	1
504			min	-.007	2	-.271	2	-.005	1	-8.179e-3	3	282.417	2	3499.332	5
505		6	max	.011	3	.258	3	.605	4	1.19e-2	2	8809.571	15	NC	1
506			min	-.007	2	-.397	2	-.002	1	-1.222e-2	3	223.682	2	3009.053	5
507		7	max	.011	3	.343	3	.578	4	1.587e-2	2	7448.517	15	NC	1
508			min	-.006	2	-.509	2	0	3	-1.626e-2	3	188.861	2	2644.889	4
509		8	max	.01	3	.412	3	.551	4	1.985e-2	2	6641.751	15	NC	1
510			min	-.006	2	-.598	2	0	12	-2.029e-2	3	168.205	2	2356.673	4
511		9	max	.01	3	.458	3	.523	4	2.228e-2	2	6218.968	15	NC	1
512			min	-.006	2	-.653	2	0	1	-2.087e-2	3	157.419	2	2148.823	4
513		10	max	.01	3	.474	3	.492	4	2.369e-2	2	6089.544	15	NC	1
514			min	-.006	2	-.672	2	0	10	-1.912e-2	3	154.264	2	2072.724	4
515		11	max	.01	3	.463	3	.458	4	2.509e-2	2	6218.567	15	NC	1
516			min	-.006	2	-.653	2	0	12	-1.737e-2	3	158.001	2	2091.032	4
517		12	max	.009	3	.424	3	.421	4	2.403e-2	2	6640.804	15	NC	1
518			min	-.006	2	-.595	2	0	1	-1.511e-2	3	169.912	2	2202.157	4
519		13	max	.009	3	.362	3	.379	4	1.927e-2	2	7446.684	15	NC	1
520			min	-.006	2	-.502	2	0	1	-1.209e-2	3	192.89	2	2580.311	4
521		14	max	.009	3	.282	3	.332	4	1.452e-2	2	8806.237	15	NC	1
522			min	-.006	2	-.386	2	0	12	-9.067e-3	3	232.104	2	3451.392	4
523		15	max	.009	3	.192	3	.284	4	9.763e-3	2	NC	15	NC	1
524			min	-.006	2	-.257	2	0	12	-6.046e-3	3	299.404	2	5549.012	4
525		16	max	.008	3	.098	3	.237	4	8.465e-3	4	NC	15	NC	1
526			min	-.006	2	-.128	2	0	12	-3.025e-3	3	423.478	2	NC	1
527		17	max	.008	3	.006	3	.194	4	9.69e-3	4	NC	5	NC	1
528			min	-.005	2	-.007	2	0	12	-3.167e-6	3	687.118	2	NC	1
529		18	max	.008	3	.094	2	.157	4	6.235e-3	2	NC	5	NC	1
530			min	-.005	2	-.077	3	0	12	-2.164e-3	3	1452.515	2	NC	1
531		19	max	.008	3	.185	2	.126	4	1.241e-2	2	NC	1	NC	1
532			min	-.005	2	-.155	3	0	1	-4.417e-3	3	NC	1	NC	1
533	M5	1	max	.034	3	.351	2	.728	4	0	1	NC	1	NC	1
534			min	-.024	2	-.018	3	0	1	-1.308e-5	4	NC	1	NC	1
535		2	max	.034	3	.168	2	.711	4	6.451e-3	4	NC	5	NC	1
536			min	-.024	2	-.003	3	0	1	0	1	747.753	2	8005.991	4
537		3	max	.034	3	.053	3	.689	4	1.276e-2	4	NC	5	NC	1
538			min	-.024	2	-.041	2	0	1	0	1	348.985	2	4750.785	4
539		4	max	.033	3	.186	3	.663	4	1.04e-2	4	9371.619	15	NC	1
540			min	-.024	2	-.294	2	0	1	0	1	211.627	2	3724.491	4
541		5	max	.033	3	.372	3	.635	4	8.032e-3	4	6506.28	15	NC	1
542			min	-.023	2	-.572	2	0	1	0	1	147.735	2	3240.894	4
543		6	max	.032	3	.583	3	.606	4	5.668e-3	4	4980.051	15	NC	1
544			min	-.023	2	-.85	2	0	1	0	1	113.486	2	2939.297	4
545		7	max	.031	3	.79	3	.577	4	3.305e-3	4	4103.926	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.023	2	-1.103	2	0	1	0	1	93.726	2	2679.59	4
547	8	max	.031	3	.964	3	.55	4	9.415e-4	4	3596.944	15	NC	1
548		min	-.022	2	-1.306	2	0	1	0	1	82.25	2	2395.766	4
549	9	max	.03	3	1.077	3	.523	4	0	1	3337.557	15	NC	1
550		min	-.022	2	-1.436	2	0	1	-8.177e-6	5	76.364	2	2140.137	4
551	10	max	.029	3	1.117	3	.491	4	0	1	3259.452	15	NC	1
552		min	-.021	2	-1.48	2	0	1	-7.892e-6	5	74.65	2	2091.026	4
553	11	max	.028	3	1.089	3	.457	4	0	1	3337.807	15	NC	1
554		min	-.021	2	-1.436	2	0	1	-7.607e-6	5	76.681	2	2123.73	4
555	12	max	.028	3	.993	3	.422	4	6.791e-4	4	3597.529	15	NC	1
556		min	-.021	2	-1.301	2	0	1	0	1	83.298	2	2158.203	4
557	13	max	.027	3	.839	3	.38	4	2.386e-3	4	4105.086	15	NC	1
558		min	-.02	2	-1.085	2	0	1	0	1	96.479	2	2510.984	4
559	14	max	.026	3	.646	3	.332	4	4.092e-3	4	4982.27	15	NC	1
560		min	-.02	2	-.819	2	0	1	0	1	119.796	2	3519.162	4
561	15	max	.025	3	.432	3	.28	4	5.799e-3	4	6510.612	15	NC	1
562		min	-.02	2	-.532	2	0	1	0	1	161.794	2	6635.901	4
563	16	max	.025	3	.217	3	.231	4	7.505e-3	4	9380.645	15	NC	1
564		min	-.019	2	-.256	2	0	1	0	1	244.331	2	NC	1
565	17	max	.024	3	.018	3	.186	4	9.212e-3	4	NC	5	NC	1
566		min	-.019	2	-.021	2	0	1	0	1	432.426	2	NC	1
567	18	max	.024	3	.148	2	.151	4	4.658e-3	4	NC	5	NC	1
568		min	-.019	2	-.15	3	0	1	0	1	981.417	2	NC	1
569	19	max	.024	3	.281	2	.127	4	0	1	NC	1	NC	1
570		min	-.019	2	-.298	3	0	1	-7.683e-6	4	NC	1	NC	1
571	M9	1	max	.011	3	.209	.727	4	1.703e-2	3	NC	1	NC	1
572		min	-.007	2	-.06	3	0	1	-7.168e-3	2	NC	1	NC	1
573	2	max	.011	3	.101	2	.709	4	8.454e-3	3	NC	5	NC	1
574		min	-.007	2	-.028	3	0	12	-3.512e-3	2	1258.061	2	8815.944	4
575	3	max	.011	3	.018	3	.687	4	1.271e-2	4	NC	5	NC	1
576		min	-.007	2	-.014	2	0	12	-1.88e-5	10	609.733	2	5100.813	4
577	4	max	.011	3	.086	3	.662	4	1.004e-2	5	NC	15	NC	1
578		min	-.007	2	-.14	2	0	12	-3.961e-3	2	388.444	2	3890.464	4
579	5	max	.011	3	.17	3	.635	4	8.179e-3	3	NC	15	NC	1
580		min	-.007	2	-.271	2	0	12	-7.932e-3	2	282.417	2	3297.486	4
581	6	max	.011	3	.258	3	.606	4	1.222e-2	3	8765.666	15	NC	1
582		min	-.007	2	-.397	2	0	10	-1.19e-2	2	223.682	2	2928.948	4
583	7	max	.011	3	.343	3	.578	4	1.626e-2	3	7412.412	15	NC	1
584		min	-.006	2	-.509	2	0	1	-1.587e-2	2	188.861	2	2642.698	4
585	8	max	.01	3	.412	3	.55	4	2.029e-2	3	6610.169	15	NC	1
586		min	-.006	2	-.598	2	0	1	-1.985e-2	2	168.205	2	2373.588	4
587	9	max	.01	3	.458	3	.523	4	2.087e-2	3	6189.695	15	NC	1
588		min	-.006	2	-.653	2	0	12	-2.228e-2	2	157.419	2	2142.207	4
589	10	max	.01	3	.474	3	.492	4	1.912e-2	3	6060.926	15	NC	1
590		min	-.006	2	-.672	2	0	1	-2.369e-2	2	154.264	2	2073.682	4
591	11	max	.01	3	.463	3	.457	4	1.737e-2	3	6189.232	15	NC	1
592		min	-.006	2	-.653	2	0	1	-2.509e-2	2	158.001	2	2098.738	4
593	12	max	.009	3	.424	3	.422	4	1.511e-2	3	6609.256	15	NC	1
594		min	-.006	2	-.595	2	0	12	-2.403e-2	2	169.912	2	2187.263	4
595	13	max	.009	3	.362	3	.379	4	1.209e-2	3	7410.946	15	NC	1
596		min	-.006	2	-.502	2	0	10	-1.927e-2	2	192.89	2	2577.832	4
597	14	max	.009	3	.282	3	.331	4	9.067e-3	3	8763.377	15	NC	1
598		min	-.006	2	-.386	2	-.002	1	-1.452e-2	2	232.104	2	3543.399	5
599	15	max	.009	3	.192	3	.281	4	6.046e-3	3	NC	15	NC	1
600		min	-.006	2	-.257	2	-.005	1	-9.763e-3	2	299.404	2	6035.973	5
601	16	max	.008	3	.098	3	.233	4	7.509e-3	5	NC	15	NC	1
602		min	-.006	2	-.128	2	-.007	1	-5.008e-3	2	423.478	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.008	3	.006	3	.189	4	9.401e-3	4	NC	5	NC	1
604		min	-.005	2	-.007	2	-.007	1	-5.009e-4	1	687.118	2	NC	1
605	18	max	.008	3	.094	2	.154	4	4.606e-3	5	NC	5	NC	1
606		min	-.005	2	-.077	3	-.005	1	-6.235e-3	2	1452.515	2	NC	1
607	19	max	.008	3	.185	2	.127	4	4.417e-3	3	NC	1	NC	1
608		min	-.005	2	-.155	3	0	12	-1.241e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

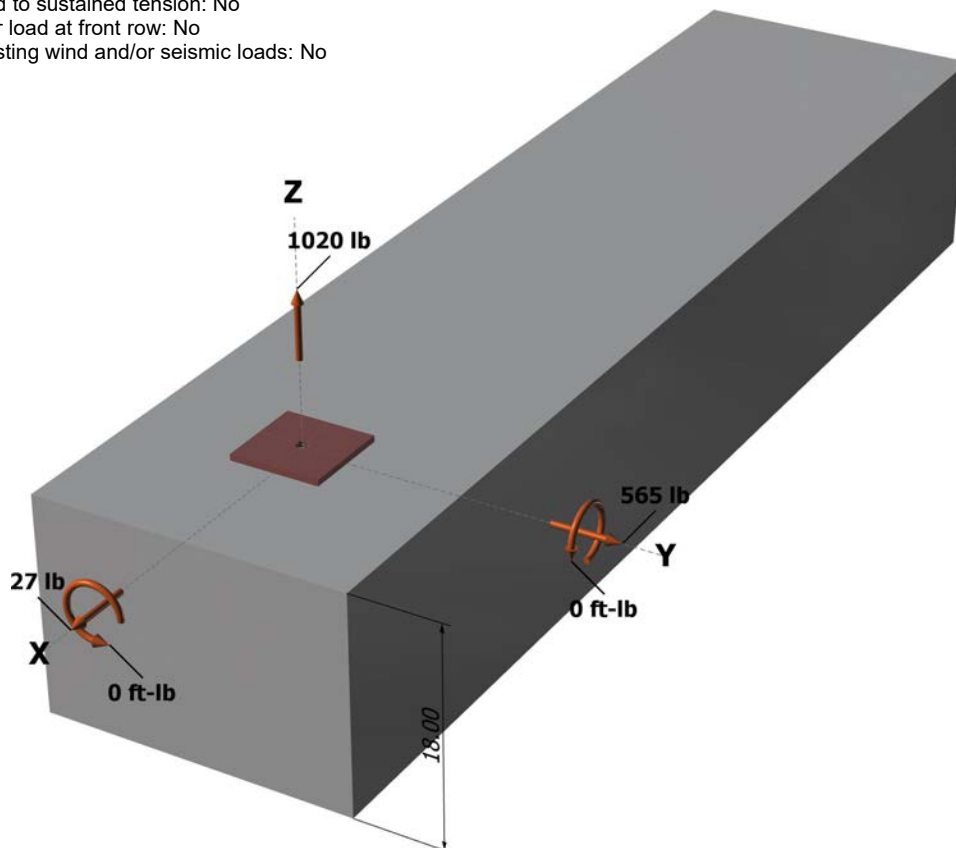
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
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Company:	Schletter, Inc.	Date:	8/1/2016
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Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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# Anchor Designer™ Software Version 2.4.6025.0

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Address:			
Phone:			
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain ( $\epsilon_o$ ): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

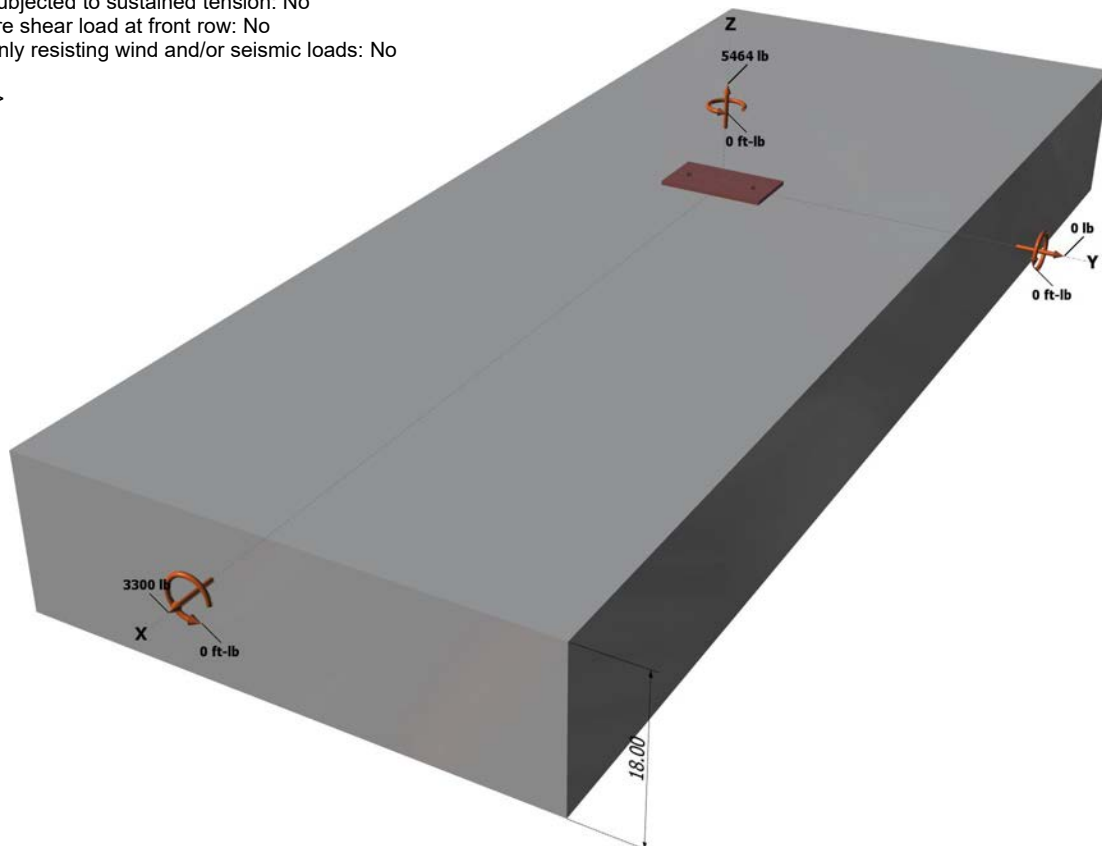
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



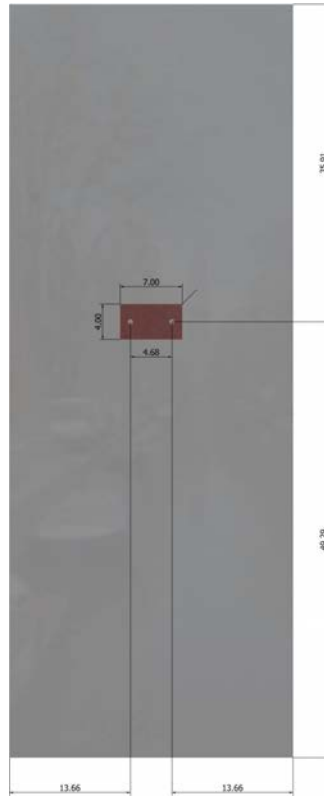
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.6025.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

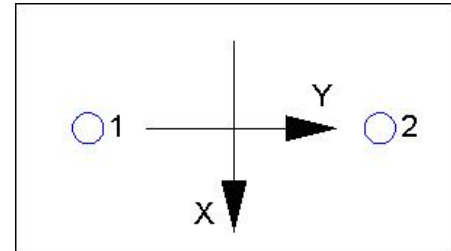
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.