

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

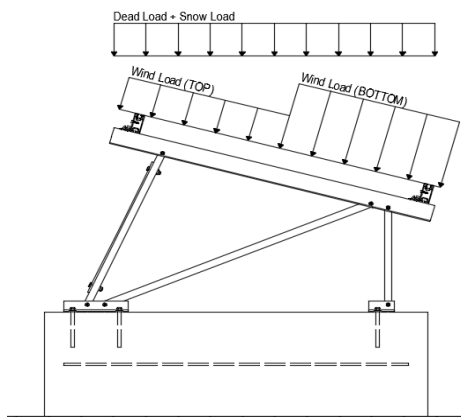
1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

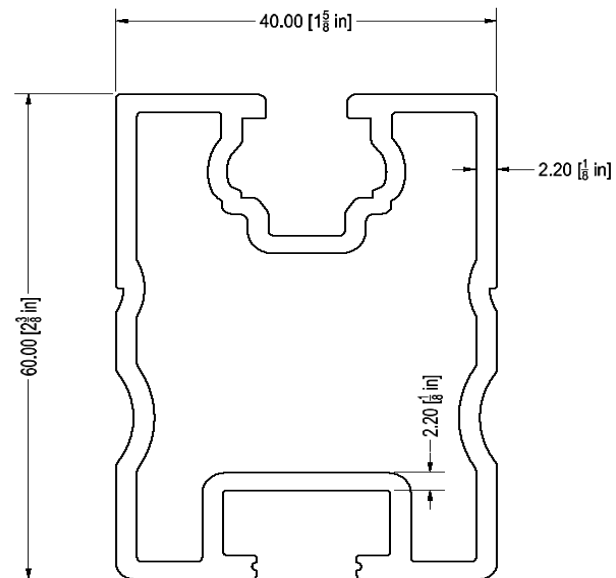
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

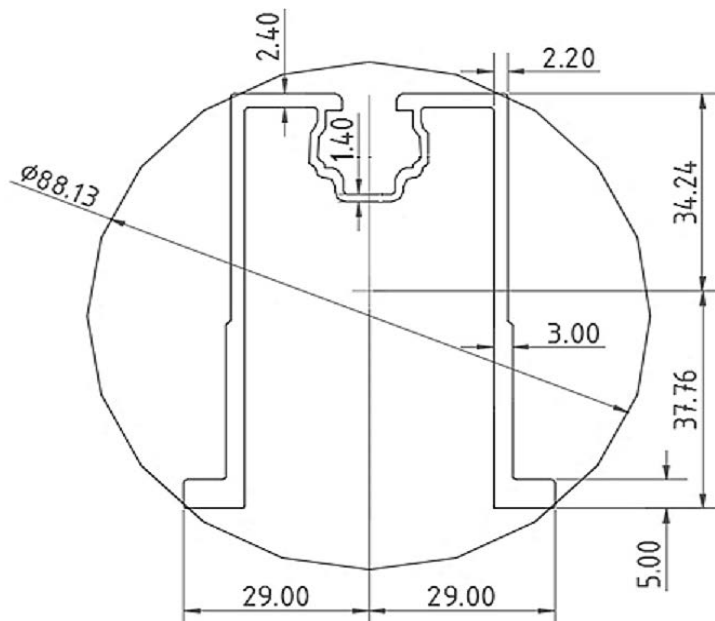
Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	90 in
ΦF_{ty} STRONG-AXIS =	28.37 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.926 k-ft
M_z =	0.205 k-ft
$M_{y \text{ allowable}}$ =	1.207 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	100%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

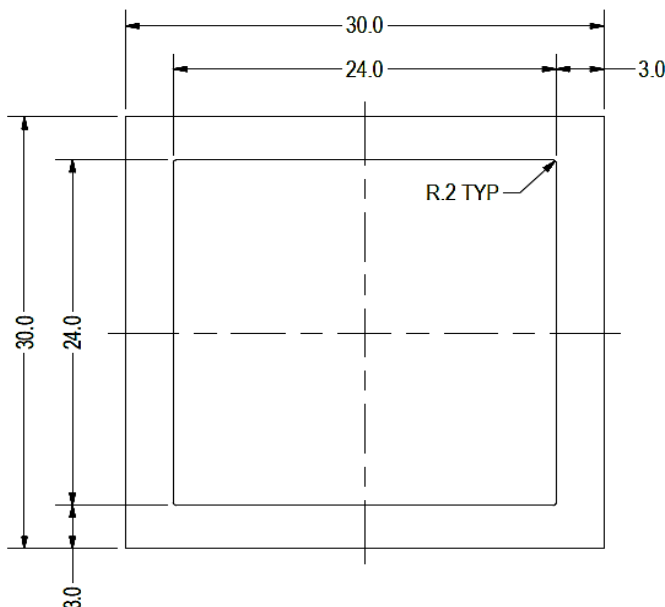
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.81 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.588 k-ft
M_z =	0.000 k-ft
P_n =	0.307 k
$M_{y \text{ allowable}}$ =	1.463 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	43%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

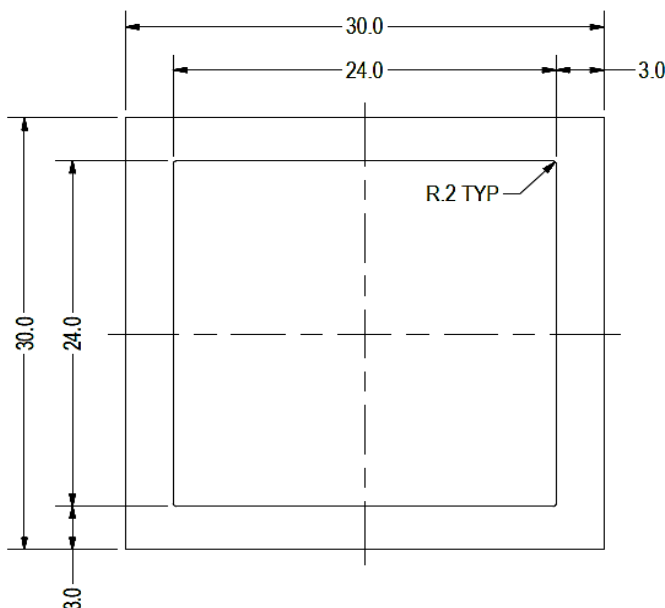
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.308 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	11%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

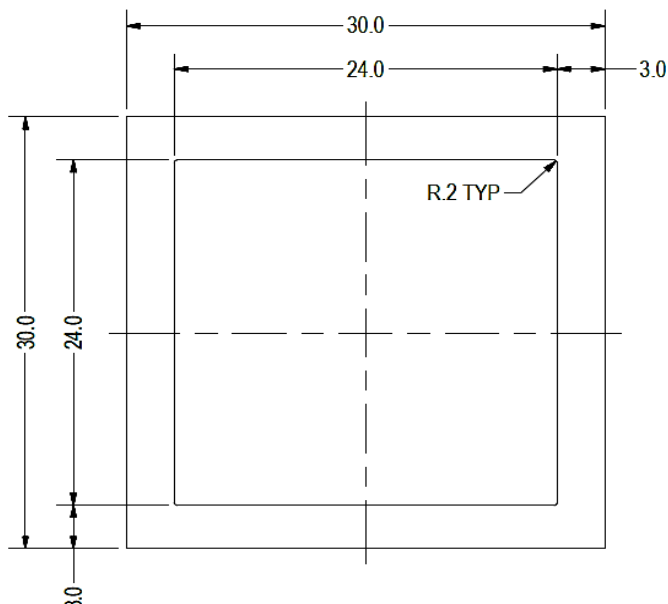
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.476 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

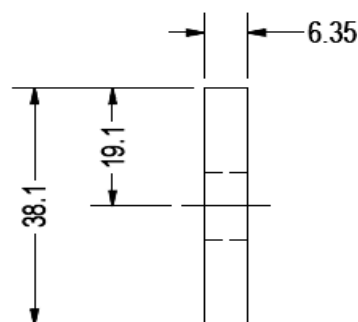
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.087 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<u>19%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.005 k-ft
P_n =	0.043 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>11%</u>



A cross brace kit is required every 17 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

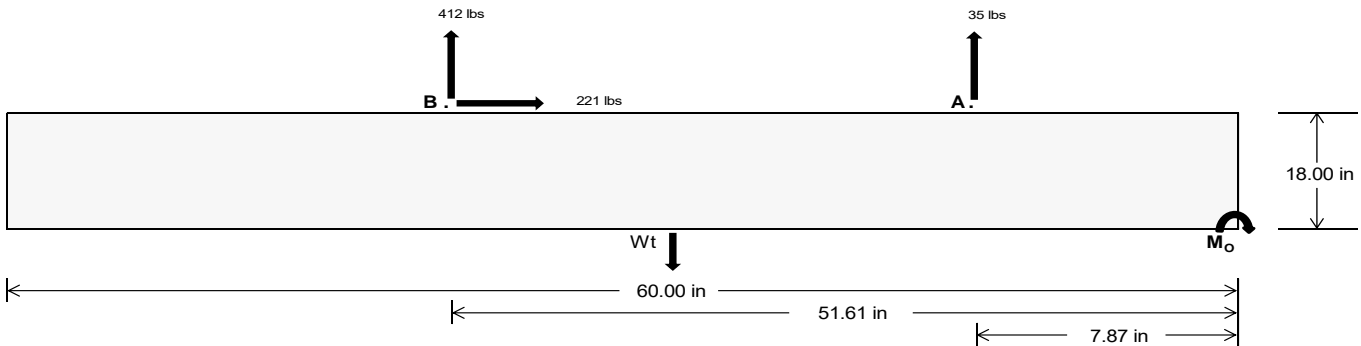
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>151.95</u>	<u>1716.66</u>	k
Compressive Load =	<u>1700.79</u>	<u>1414.78</u>	k
Lateral Load =	<u>4.73</u>	<u>921.21</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 25517.5$ in-lbs
Resisting Force Required = 850.58 lbs
S.F. = 1.67
Weight Required = 1417.64 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 221.34 lbs
Friction = 0.4
Weight Required = 553.36 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 221.34 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	651 lbs	651 lbs	651 lbs	651 lbs	493 lbs	493 lbs	493 lbs	493 lbs	805 lbs	805 lbs	805 lbs	805 lbs	-70 lbs	-70 lbs	-70 lbs	-70 lbs
F_B	472 lbs	472 lbs	472 lbs	472 lbs	533 lbs	533 lbs	533 lbs	533 lbs	715 lbs	715 lbs	715 lbs	715 lbs	-824 lbs	-824 lbs	-824 lbs	-824 lbs
F_V	72 lbs	72 lbs	72 lbs	72 lbs	402 lbs	402 lbs	402 lbs	402 lbs	350 lbs	350 lbs	350 lbs	350 lbs	-443 lbs	-443 lbs	-443 lbs	-443 lbs
P_{total}	3117 lbs	3207 lbs	3298 lbs	3389 lbs	3020 lbs	3111 lbs	3201 lbs	3292 lbs	3513 lbs	3604 lbs	3695 lbs	3785 lbs	302 lbs	356 lbs	411 lbs	465 lbs
M	457 lbs-ft	457 lbs-ft	457 lbs-ft	457 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	722 lbs-ft	722 lbs-ft	722 lbs-ft	722 lbs-ft	690 lbs-ft	690 lbs-ft	690 lbs-ft	690 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.28 ft	1.93 ft	1.68 ft	1.48 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	280.1 psf	277.4 psf	274.9 psf	272.6 psf	257.3 psf	255.6 psf	254.0 psf	252.5 psf	288.8 psf	285.7 psf	282.8 psf	280.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	399.9 psf	391.9 psf	384.7 psf	378.0 psf	401.6 psf	393.6 psf	386.3 psf	379.5 psf	477.8 psf	466.5 psf	456.1 psf	446.6 psf	504.8 psf	219.2 psf	166.7 psf	146.3 psf

Maximum Bearing Pressure = 505 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

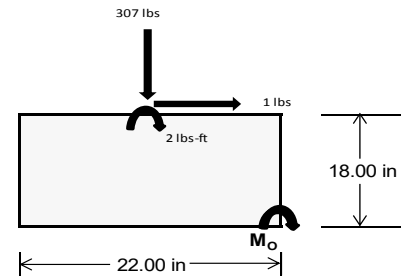
Overturning Check

$M_o = 277.8 \text{ ft-lbs}$
 Resisting Force Required = 303.04 lbs
 S.F. = 1.67
 Weight Required = 505.07 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	85 lbs	228 lbs	81 lbs	312 lbs	921 lbs	307 lbs	25 lbs	67 lbs	24 lbs
F_v	4 lbs	4 lbs	0 lbs	19 lbs	18 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2554 lbs	2696 lbs	2549 lbs	2661 lbs	3271 lbs	2657 lbs	747 lbs	788 lbs	745 lbs
M	6 lbs-ft	6 lbs-ft	0 lbs-ft	33 lbs-ft	27 lbs-ft	4 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.81 ft	1.82 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	276.3 sqft	292.0 sqft	277.9 sqft	278.6 sqft	347.1 sqft	288.5 sqft	80.8 sqft	85.4 sqft	81.3 sqft
f_{max}	280.9 psf	296.3 psf	278.2 psf	302.0 psf	366.5 psf	291.1 psf	82.1 psf	86.6 psf	81.4 psf



Maximum Bearing Pressure = 367 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.415 k
Allowable Uplift =	1.214 k
Utilization =	<u>34%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.308 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

Diagonal Strut

Maximum Axial Load =	0.476 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.184 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.043 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.059 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 90.00 \text{ in}$$

$$J = 0.255$$

$$234.355$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 90.00 \text{ in}$$

$$J = 0.255$$

$$243.363$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.4 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.207 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &21.0912 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

3.4.15

N/A for Strong Direction

3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &24.5845 \end{aligned}$$

$$S1 = \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.8 \text{ ksi}$$

3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

3.4.16

N/A for Weak Direction

3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{maxSt} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{maxWk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.5514 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.7972 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 11.5927 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

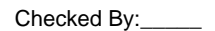
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 11.59 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.82 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 25° 90mph 30psf 7.5ft 7-05 N8 Page 120



Company : Schletter, Inc.
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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	169.386	2	295.899	2	-.001	15	0	2	0	1	0	1
2		min	-221.123	3	-400.159	3	-.158	1	0	3	0	1	0	1
3	N7	max	0	15	479.645	1	-.071	15	0	15	0	1	0	1
4		min	-.163	2	-26.56	3	-1.685	1	-.003	1	0	1	0	1
5	N15	max	0	15	1308.301	1	.602	1	.001	1	0	1	0	1
6		min	-1.878	1	-116.887	3	-.349	3	0	3	0	1	0	1
7	N16	max	670.061	2	1088.29	1	-.184	10	0	1	0	1	0	1
8		min	-708.622	3	-1320.504	3	-38.983	3	0	3	0	1	0	1
9	N23	max	0	15	479.342	1	3.635	1	.006	1	0	1	0	1
10		min	-.163	2	-26.077	3	.143	15	0	15	0	1	0	1
11	N24	max	169.879	2	300.352	2	39.259	3	.002	1	0	1	0	1
12		min	-221.218	3	-397.52	3	.027	10	0	3	0	1	0	1
13	Totals:	max	1007.245	2	3946.105	1	0	1						
14		min	-1151.131	3	-2287.706	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	331.882	1	.641	4	.659	1	0	15	0	3	0	1
2			min	-361.109	3	.151	15	-.046	3	-.001	1	0	1	0	1
3		2	max	331.998	1	.596	4	.659	1	0	15	0	3	0	15
4			min	-361.021	3	.141	15	-.046	3	-.001	1	0	1	0	4
5		3	max	332.115	1	.55	4	.659	1	0	15	0	1	0	15
6			min	-360.934	3	.13	15	-.046	3	-.001	1	0	10	0	4
7		4	max	332.231	1	.504	4	.659	1	0	15	0	1	0	15
8			min	-360.847	3	.119	15	-.046	3	-.001	1	0	3	0	4
9		5	max	332.347	1	.459	4	.659	1	0	15	0	1	0	15
10			min	-360.759	3	.108	15	-.046	3	-.001	1	0	3	0	4
11		6	max	332.464	1	.413	4	.659	1	0	15	0	1	0	15
12			min	-360.672	3	.098	15	-.046	3	-.001	1	0	3	0	4
13		7	max	332.58	1	.367	4	.659	1	0	15	0	1	0	15
14			min	-360.585	3	.087	15	-.046	3	-.001	1	0	3	0	4
15		8	max	332.697	1	.322	4	.659	1	0	15	0	1	0	15
16			min	-360.498	3	.076	15	-.046	3	-.001	1	0	3	0	4
17		9	max	332.813	1	.276	4	.659	1	0	15	0	1	0	15
18			min	-360.41	3	.065	15	-.046	3	-.001	1	0	3	0	4
19		10	max	332.929	1	.23	4	.659	1	0	15	0	1	0	15
20			min	-360.323	3	.055	15	-.046	3	-.001	1	0	3	0	4
21		11	max	333.046	1	.185	4	.659	1	0	15	0	1	0	15
22			min	-360.236	3	.044	15	-.046	3	-.001	1	0	3	0	4
23		12	max	333.162	1	.139	4	.659	1	0	15	.001	1	0	15
24			min	-360.148	3	.033	15	-.046	3	-.001	1	0	3	0	4
25		13	max	333.279	1	.099	2	.659	1	0	15	.001	1	0	15
26			min	-360.061	3	.017	12	-.046	3	-.001	1	0	3	0	4
27		14	max	333.395	1	.063	2	.659	1	0	15	.001	1	0	15
28			min	-359.974	3	-.002	3	-.046	3	-.001	1	0	3	0	4
29		15	max	333.511	1	.028	2	.659	1	0	15	.001	1	0	15
30			min	-359.886	3	-.029	3	-.046	3	-.001	1	0	3	0	4
31		16	max	333.628	1	-.007	10	.659	1	0	15	.001	1	0	15
32			min	-359.799	3	-.056	3	-.046	3	-.001	1	0	3	0	4
33		17	max	333.744	1	-.02	15	.659	1	0	15	.002	1	0	15
34			min	-359.712	3	-.089	4	-.046	3	-.001	1	0	3	0	4
35		18	max	333.861	1	-.031	15	.659	1	0	15	.002	1	0	15
36			min	-359.625	3	-.135	4	-.046	3	-.001	1	0	3	0	4
37		19	max	333.977	1	-.042	15	.659	1	0	15	.002	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-359.537	3	-.181	4	-.046	3	-.001	1	0	3	0	4
39	M3	1	max	105.51	2	1.777	4	-.025	15	0	.002	1	0	4
40		min	-128.812	3	.418	15	-.671	1	0	1	0	15	0	15
41		2	max	105.441	2	1.599	4	-.025	15	0	.002	1	0	2
42		min	-128.863	3	.376	15	-.671	1	0	1	0	15	0	12
43		3	max	105.372	2	1.422	4	-.025	15	0	.002	1	0	2
44		min	-128.915	3	.335	15	-.671	1	0	1	0	15	0	3
45		4	max	105.304	2	1.245	4	-.025	15	0	.002	1	0	15
46		min	-128.966	3	.293	15	-.671	1	0	1	0	15	0	4
47		5	max	105.235	2	1.068	4	-.025	15	0	.002	1	0	15
48		min	-129.018	3	.251	15	-.671	1	0	1	0	15	0	4
49		6	max	105.167	2	.891	4	-.025	15	0	.001	1	0	15
50		min	-129.069	3	.21	15	-.671	1	0	1	0	15	0	4
51		7	max	105.098	2	.713	4	-.025	15	0	.001	1	0	15
52		min	-129.121	3	.168	15	-.671	1	0	1	0	15	0	4
53		8	max	105.029	2	.536	4	-.025	15	0	.001	1	0	15
54		min	-129.172	3	.127	15	-.671	1	0	1	0	15	-.001	4
55		9	max	104.961	2	.359	4	-.025	15	0	0	1	0	15
56		min	-129.223	3	.085	15	-.671	1	0	1	0	15	-.001	4
57		10	max	104.892	2	.182	4	-.025	15	0	0	1	0	15
58		min	-129.275	3	.043	15	-.671	1	0	1	0	15	-.001	4
59		11	max	104.824	2	.024	2	-.025	15	0	0	1	0	15
60		min	-129.326	3	-.021	3	-.671	1	0	1	0	15	-.001	4
61		12	max	104.755	2	-.04	15	-.025	15	0	0	1	0	15
62		min	-129.378	3	-.173	4	-.671	1	0	1	0	15	-.001	4
63		13	max	104.686	2	-.082	15	-.025	15	0	0	1	0	15
64		min	-129.429	3	-.35	4	-.671	1	0	1	0	15	-.001	4
65		14	max	104.618	2	-.123	15	-.025	15	0	0	1	0	15
66		min	-129.481	3	-.527	4	-.671	1	0	1	0	12	-.001	4
67		15	max	104.549	2	-.165	15	-.025	15	0	0	1	0	15
68		min	-129.532	3	-.704	4	-.671	1	0	1	0	3	0	4
69		16	max	104.481	2	-.207	15	-.025	15	0	0	15	0	15
70		min	-129.584	3	-.881	4	-.671	1	0	1	0	1	0	4
71		17	max	104.412	2	-.248	15	-.025	15	0	0	15	0	15
72		min	-129.635	3	-1.059	4	-.671	1	0	1	0	1	0	4
73		18	max	104.343	2	-.29	15	-.025	15	0	0	15	0	15
74		min	-129.687	3	-1.236	4	-.671	1	0	1	0	1	0	4
75		19	max	104.275	2	-.332	15	-.025	15	0	0	15	0	1
76		min	-129.738	3	-1.413	4	-.671	1	0	1	0	1	0	1
77	M4	1	max	478.48	1	0	1	-.071	15	0	0	3	0	1
78		min	-27.433	3	0	1	-1.832	1	0	1	0	1	0	1
79		2	max	478.545	1	0	1	-.071	15	0	0	12	0	1
80		min	-27.385	3	0	1	-1.832	1	0	1	0	1	0	1
81		3	max	478.61	1	0	1	-.071	15	0	0	15	0	1
82		min	-27.336	3	0	1	-1.832	1	0	1	0	1	0	1
83		4	max	478.675	1	0	1	-.071	15	0	0	15	0	1
84		min	-27.287	3	0	1	-1.832	1	0	1	0	1	0	1
85		5	max	478.739	1	0	1	-.071	15	0	0	15	0	1
86		min	-27.239	3	0	1	-1.832	1	0	1	0	1	0	1
87		6	max	478.804	1	0	1	-.071	15	0	0	15	0	1
88		min	-27.19	3	0	1	-1.832	1	0	1	0	1	0	1
89		7	max	478.869	1	0	1	-.071	15	0	0	15	0	1
90		min	-27.142	3	0	1	-1.832	1	0	1	-.001	1	0	1
91		8	max	478.933	1	0	1	-.071	15	0	0	15	0	1
92		min	-27.093	3	0	1	-1.832	1	0	1	-.001	1	0	1
93		9	max	478.998	1	0	1	-.071	15	0	0	15	0	1
94		min	-27.045	3	0	1	-1.832	1	0	1	-.001	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	479.063	1	0	1	-.071	15	0	1	0	15	0	1
96		min	-26.996	3	0	1	-1.832	1	0	1	-.002	1	0	1
97	11	max	479.127	1	0	1	-.071	15	0	1	0	15	0	1
98		min	-26.948	3	0	1	-1.832	1	0	1	-.002	1	0	1
99	12	max	479.192	1	0	1	-.071	15	0	1	0	15	0	1
100		min	-26.899	3	0	1	-1.832	1	0	1	-.002	1	0	1
101	13	max	479.257	1	0	1	-.071	15	0	1	0	15	0	1
102		min	-26.851	3	0	1	-1.832	1	0	1	-.002	1	0	1
103	14	max	479.322	1	0	1	-.071	15	0	1	0	15	0	1
104		min	-26.802	3	0	1	-1.832	1	0	1	-.002	1	0	1
105	15	max	479.386	1	0	1	-.071	15	0	1	0	15	0	1
106		min	-26.754	3	0	1	-1.832	1	0	1	-.002	1	0	1
107	16	max	479.451	1	0	1	-.071	15	0	1	0	15	0	1
108		min	-26.705	3	0	1	-1.832	1	0	1	-.003	1	0	1
109	17	max	479.516	1	0	1	-.071	15	0	1	0	15	0	1
110		min	-26.657	3	0	1	-1.832	1	0	1	-.003	1	0	1
111	18	max	479.58	1	0	1	-.071	15	0	1	0	15	0	1
112		min	-26.608	3	0	1	-1.832	1	0	1	-.003	1	0	1
113	19	max	479.645	1	0	1	-.071	15	0	1	0	15	0	1
114		min	-26.56	3	0	1	-1.832	1	0	1	-.003	1	0	1
115	M6	1	max 1085.263	1	.642	4	.216	1	0	1	0	3	0	1
116		min	-1184.471	3	.151	15	-.135	3	0	15	0	1	0	1
117	2	max	1085.379	1	.596	4	.216	1	0	1	0	3	0	15
118		min	-1184.383	3	.141	15	-.135	3	0	15	0	11	0	4
119	3	max	1085.496	1	.55	4	.216	1	0	1	0	3	0	15
120		min	-1184.296	3	.13	15	-.135	3	0	15	0	15	0	4
121	4	max	1085.612	1	.505	4	.216	1	0	1	0	1	0	15
122		min	-1184.209	3	.119	15	-.135	3	0	15	0	15	0	4
123	5	max	1085.729	1	.459	4	.216	1	0	1	0	1	0	15
124		min	-1184.121	3	.108	15	-.135	3	0	15	0	12	0	4
125	6	max	1085.845	1	.415	2	.216	1	0	1	0	1	0	15
126		min	-1184.034	3	.098	15	-.135	3	0	15	0	3	0	4
127	7	max	1085.961	1	.379	2	.216	1	0	1	0	1	0	15
128		min	-1183.947	3	.082	12	-.135	3	0	15	0	3	0	4
129	8	max	1086.078	1	.344	2	.216	1	0	1	0	1	0	15
130		min	-1183.86	3	.064	12	-.135	3	0	15	0	3	0	4
131	9	max	1086.194	1	.308	2	.216	1	0	1	0	1	0	15
132		min	-1183.772	3	.047	12	-.135	3	0	15	0	3	0	4
133	10	max	1086.311	1	.272	2	.216	1	0	1	0	1	0	15
134		min	-1183.685	3	.029	12	-.135	3	0	15	0	3	0	4
135	11	max	1086.427	1	.237	2	.216	1	0	1	0	1	0	15
136		min	-1183.598	3	.011	3	-.135	3	0	15	0	3	0	2
137	12	max	1086.543	1	.201	2	.216	1	0	1	0	1	0	12
138		min	-1183.51	3	-.016	3	-.135	3	0	15	0	3	0	2
139	13	max	1086.66	1	.166	2	.216	1	0	1	0	1	0	12
140		min	-1183.423	3	-.043	3	-.135	3	0	15	0	3	0	2
141	14	max	1086.776	1	.13	2	.216	1	0	1	0	1	0	12
142		min	-1183.336	3	-.07	3	-.135	3	0	15	0	3	0	2
143	15	max	1086.893	1	.095	2	.216	1	0	1	0	1	0	12
144		min	-1183.248	3	-.096	3	-.135	3	0	15	0	3	0	2
145	16	max	1087.009	1	.059	2	.216	1	0	1	0	1	0	12
146		min	-1183.161	3	-.123	3	-.135	3	0	15	0	3	0	2
147	17	max	1087.125	1	.023	2	.216	1	0	1	0	1	0	12
148		min	-1183.074	3	-.15	3	-.135	3	0	15	0	3	0	2
149	18	max	1087.242	1	-.012	2	.216	1	0	1	0	1	0	12
150		min	-1182.987	3	-.176	3	-.135	3	0	15	0	3	0	2
151	19	max	1087.358	1	-.042	15	.216	1	0	1	0	1	0	12



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1182.899	3	-.203	3	-.135	3	0	15	0	3	0	2
153	M7	1	max	475.718	2	1.779	4	.014	1	0	2	0	2	2
154		min	-398.415	3	.419	15	-.004	10	0	3	0	3	0	12
155		2	max	475.649	2	1.602	4	.014	1	0	2	0	2	2
156		min	-398.466	3	.377	15	-.004	10	0	3	0	3	0	3
157		3	max	475.581	2	1.425	4	.014	1	0	2	0	2	2
158		min	-398.518	3	.335	15	-.004	10	0	3	0	3	0	3
159		4	max	475.512	2	1.248	4	.014	1	0	2	0	2	2
160		min	-398.569	3	.294	15	-.004	10	0	3	0	3	0	3
161		5	max	475.443	2	1.07	4	.014	1	0	2	0	2	15
162		min	-398.621	3	.252	15	-.004	10	0	3	0	3	0	3
163		6	max	475.375	2	.893	4	.014	1	0	2	0	2	15
164		min	-398.672	3	.21	15	-.004	10	0	3	0	3	0	4
165		7	max	475.306	2	.716	4	.014	1	0	2	0	2	15
166		min	-398.724	3	.169	15	-.004	10	0	3	0	3	0	4
167		8	max	475.238	2	.539	4	.014	1	0	2	0	2	15
168		min	-398.775	3	.127	15	-.004	10	0	3	0	3	-.001	4
169		9	max	475.169	2	.362	2	.014	1	0	2	0	2	15
170		min	-398.827	3	.085	15	-.004	10	0	3	0	3	-.001	4
171		10	max	475.1	2	.224	2	.014	1	0	2	0	2	15
172		min	-398.878	3	.018	12	-.004	10	0	3	0	3	-.001	4
173		11	max	475.032	2	.086	2	.014	1	0	2	0	2	15
174		min	-398.93	3	-.082	3	-.004	10	0	3	0	3	-.001	4
175		12	max	474.963	2	-.04	15	.014	1	0	2	0	2	15
176		min	-398.981	3	-.186	3	-.004	10	0	3	0	3	-.001	4
177		13	max	474.895	2	-.081	15	.014	1	0	2	0	2	15
178		min	-399.032	3	-.347	4	-.004	10	0	3	0	3	-.001	4
179		14	max	474.826	2	-.123	15	.014	1	0	2	0	2	15
180		min	-399.084	3	-.524	4	-.004	10	0	3	0	3	-.001	4
181		15	max	474.757	2	-.165	15	.014	1	0	2	0	2	15
182		min	-399.135	3	-.702	4	-.004	10	0	3	0	3	0	4
183		16	max	474.689	2	-.206	15	.014	1	0	2	0	2	15
184		min	-399.187	3	-.879	4	-.004	10	0	3	0	3	0	4
185		17	max	474.62	2	-.248	15	.014	1	0	2	0	2	15
186		min	-399.238	3	-1.056	4	-.004	10	0	3	0	3	0	4
187		18	max	474.552	2	-.29	15	.014	1	0	2	0	2	15
188		min	-399.29	3	-1.233	4	-.004	10	0	3	0	3	0	4
189		19	max	474.483	2	-.331	15	.014	1	0	2	0	2	1
190		min	-399.341	3	-1.41	4	-.004	10	0	3	0	3	0	1
191	M8	1	max	1307.136	1	0	1	.766	1	0	1	0	15	0
192		min	-117.761	3	0	1	-.342	3	0	1	0	1	0	1
193		2	max	1307.201	1	0	1	.766	1	0	1	0	1	0
194		min	-117.712	3	0	1	-.342	3	0	1	0	3	0	1
195		3	max	1307.266	1	0	1	.766	1	0	1	0	1	0
196		min	-117.664	3	0	1	-.342	3	0	1	0	3	0	1
197		4	max	1307.33	1	0	1	.766	1	0	1	0	1	0
198		min	-117.615	3	0	1	-.342	3	0	1	0	3	0	1
199		5	max	1307.395	1	0	1	.766	1	0	1	0	1	0
200		min	-117.566	3	0	1	-.342	3	0	1	0	3	0	1
201		6	max	1307.46	1	0	1	.766	1	0	1	0	1	0
202		min	-117.518	3	0	1	-.342	3	0	1	0	3	0	1
203		7	max	1307.525	1	0	1	.766	1	0	1	0	1	0
204		min	-117.469	3	0	1	-.342	3	0	1	0	3	0	1
205		8	max	1307.589	1	0	1	.766	1	0	1	0	1	0
206		min	-117.421	3	0	1	-.342	3	0	1	0	3	0	1
207		9	max	1307.654	1	0	1	.766	1	0	1	0	1	0
208		min	-117.372	3	0	1	-.342	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1307.719	1	0	1	.766	1	0	1	0	1	0	1
210			min	-117.324	3	0	1	-.342	3	0	1	0	3	0	1
211		11	max	1307.783	1	0	1	.766	1	0	1	0	1	0	1
212			min	-117.275	3	0	1	-.342	3	0	1	0	3	0	1
213		12	max	1307.848	1	0	1	.766	1	0	1	0	1	0	1
214			min	-117.227	3	0	1	-.342	3	0	1	0	3	0	1
215		13	max	1307.913	1	0	1	.766	1	0	1	0	1	0	1
216			min	-117.178	3	0	1	-.342	3	0	1	0	3	0	1
217		14	max	1307.977	1	0	1	.766	1	0	1	0	1	0	1
218			min	-117.13	3	0	1	-.342	3	0	1	0	3	0	1
219		15	max	1308.042	1	0	1	.766	1	0	1	0	1	0	1
220			min	-117.081	3	0	1	-.342	3	0	1	0	3	0	1
221		16	max	1308.107	1	0	1	.766	1	0	1	.001	1	0	1
222			min	-117.033	3	0	1	-.342	3	0	1	0	3	0	1
223		17	max	1308.172	1	0	1	.766	1	0	1	.001	1	0	1
224			min	-116.984	3	0	1	-.342	3	0	1	0	3	0	1
225		18	max	1308.236	1	0	1	.766	1	0	1	.001	1	0	1
226			min	-116.936	3	0	1	-.342	3	0	1	0	3	0	1
227		19	max	1308.301	1	0	1	.766	1	0	1	.001	1	0	1
228			min	-116.887	3	0	1	-.342	3	0	1	0	3	0	1
229	M10	1	max	344.749	1	.634	4	-.007	15	.001	1	0	1	0	1
230			min	-343.886	3	.15	15	-.188	1	0	3	0	3	0	1
231		2	max	344.865	1	.588	4	-.007	15	.001	1	0	1	0	15
232			min	-343.799	3	.139	15	-.188	1	0	3	0	3	0	4
233		3	max	344.981	1	.542	4	-.007	15	.001	1	0	1	0	15
234			min	-343.711	3	.129	15	-.188	1	0	3	0	3	0	4
235		4	max	345.098	1	.497	4	-.007	15	.001	1	0	1	0	15
236			min	-343.624	3	.118	15	-.188	1	0	3	0	3	0	4
237		5	max	345.214	1	.451	4	-.007	15	.001	1	0	1	0	15
238			min	-343.537	3	.107	15	-.188	1	0	3	0	3	0	4
239		6	max	345.331	1	.405	4	-.007	15	.001	1	0	1	0	15
240			min	-343.45	3	.097	15	-.188	1	0	3	0	3	0	4
241		7	max	345.447	1	.36	4	-.007	15	.001	1	0	1	0	15
242			min	-343.362	3	.086	15	-.188	1	0	3	0	3	0	4
243		8	max	345.563	1	.314	4	-.007	15	.001	1	0	1	0	15
244			min	-343.275	3	.075	15	-.188	1	0	3	0	3	0	4
245		9	max	345.68	1	.268	4	-.007	15	.001	1	0	11	0	15
246			min	-343.188	3	.064	15	-.188	1	0	3	0	3	0	4
247		10	max	345.796	1	.223	4	-.007	15	.001	1	0	15	0	15
248			min	-343.1	3	.054	15	-.188	1	0	3	0	3	0	4
249		11	max	345.913	1	.177	4	-.007	15	.001	1	0	15	0	15
250			min	-343.013	3	.043	15	-.188	1	0	3	0	3	0	4
251		12	max	346.029	1	.134	2	-.007	15	.001	1	0	15	0	15
252			min	-342.926	3	.032	15	-.188	1	0	3	0	3	0	4
253		13	max	346.145	1	.099	2	-.007	15	.001	1	0	15	0	15
254			min	-342.838	3	.021	15	-.188	1	0	3	0	1	0	4
255		14	max	346.262	1	.063	2	-.007	15	.001	1	0	15	0	15
256			min	-342.751	3	-.004	1	-.188	1	0	3	0	1	0	4
257		15	max	346.378	1	.028	2	-.007	15	.001	1	0	15	0	15
258			min	-342.664	3	-.04	1	-.188	1	0	3	0	1	0	4
259		16	max	346.495	1	-.008	10	-.007	15	.001	1	0	15	0	15
260			min	-342.577	3	-.076	1	-.188	1	0	3	0	1	0	4
261		17	max	346.611	1	-.022	15	-.007	15	.001	1	0	15	0	15
262			min	-342.489	3	-.111	1	-.188	1	0	3	0	1	0	4
263		18	max	346.727	1	-.032	15	-.007	15	.001	1	0	15	0	15
264			min	-342.402	3	-.147	1	-.188	1	0	3	0	1	0	4
265		19	max	346.844	1	-.043	15	-.007	15	.001	1	0	15	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-342.315	3	-.188	4	-.188	1	0	3	0	1	0	4
267	M11	1	max	105.247	2	1.781	4	.775	1	.001	1	0	3	0	4
268			min	-129.436	3	.419	15	.01	12	0	15	-.002	1	0	15
269		2	max	105.178	2	1.604	4	.775	1	.001	1	0	3	0	4
270			min	-129.487	3	.377	15	.01	12	0	15	-.002	1	0	12
271		3	max	105.109	2	1.427	4	.775	1	.001	1	0	3	0	1
272			min	-129.539	3	.335	15	.01	12	0	15	-.002	1	0	3
273		4	max	105.041	2	1.25	4	.775	1	.001	1	0	3	0	15
274			min	-129.59	3	.294	15	.01	12	0	15	-.002	1	0	3
275		5	max	104.972	2	1.073	4	.775	1	.001	1	0	3	0	15
276			min	-129.642	3	.252	15	.01	12	0	15	-.001	1	0	4
277		6	max	104.904	2	.895	4	.775	1	.001	1	0	3	0	15
278			min	-129.693	3	.211	15	.01	12	0	15	-.001	1	0	4
279		7	max	104.835	2	.718	4	.775	1	.001	1	0	3	0	15
280			min	-129.745	3	.169	15	.01	12	0	15	-.001	1	0	4
281		8	max	104.766	2	.541	4	.775	1	.001	1	0	3	0	15
282			min	-129.796	3	.127	15	.01	12	0	15	0	1	-.001	4
283		9	max	104.698	2	.364	4	.775	1	.001	1	0	3	0	15
284			min	-129.847	3	.086	15	.01	12	0	15	0	1	-.001	4
285		10	max	104.629	2	.187	4	.775	1	.001	1	0	3	0	15
286			min	-129.899	3	.044	15	.01	12	0	15	0	1	-.001	4
287		11	max	104.561	2	.025	1	.775	1	.001	1	0	3	0	15
288			min	-129.95	3	-.04	3	.01	12	0	15	0	1	-.001	4
289		12	max	104.492	2	-.039	15	.775	1	.001	1	0	3	0	15
290			min	-130.002	3	-.168	4	.01	12	0	15	0	1	-.001	4
291		13	max	104.423	2	-.081	15	.775	1	.001	1	0	3	0	15
292			min	-130.053	3	-.345	4	.01	12	0	15	0	1	-.001	4
293		14	max	104.355	2	-.123	15	.775	1	.001	1	0	3	0	15
294			min	-130.105	3	-.522	4	.01	12	0	15	0	2	-.001	4
295		15	max	104.286	2	-.164	15	.775	1	.001	1	0	3	0	15
296			min	-130.156	3	-.699	4	.01	12	0	15	0	10	0	4
297		16	max	104.218	2	-.206	15	.775	1	.001	1	0	1	0	15
298			min	-130.208	3	-.877	4	.01	12	0	15	0	15	0	4
299		17	max	104.149	2	-.248	15	.775	1	.001	1	0	1	0	15
300			min	-130.259	3	-1.054	4	.01	12	0	15	0	15	0	4
301		18	max	104.08	2	-.289	15	.775	1	.001	1	0	1	0	15
302			min	-130.311	3	-1.231	4	.01	12	0	15	0	15	0	4
303		19	max	104.012	2	-.331	15	.775	1	.001	1	0	1	0	1
304			min	-130.362	3	-1.408	4	.01	12	0	15	0	15	0	1
305	M12	1	max	478.177	1	0	1	3.947	1	0	1	0	1	0	1
306			min	-26.95	3	0	1	.144	15	0	1	0	3	0	1
307		2	max	478.242	1	0	1	3.947	1	0	1	0	1	0	1
308			min	-26.902	3	0	1	.144	15	0	1	0	15	0	1
309		3	max	478.307	1	0	1	3.947	1	0	1	0	1	0	1
310			min	-26.853	3	0	1	.144	15	0	1	0	15	0	1
311		4	max	478.371	1	0	1	3.947	1	0	1	.001	1	0	1
312			min	-26.805	3	0	1	.144	15	0	1	0	15	0	1
313		5	max	478.436	1	0	1	3.947	1	0	1	.001	1	0	1
314			min	-26.756	3	0	1	.144	15	0	1	0	15	0	1
315		6	max	478.501	1	0	1	3.947	1	0	1	.002	1	0	1
316			min	-26.708	3	0	1	.144	15	0	1	0	15	0	1
317		7	max	478.565	1	0	1	3.947	1	0	1	.002	1	0	1
318			min	-26.659	3	0	1	.144	15	0	1	0	15	0	1
319		8	max	478.63	1	0	1	3.947	1	0	1	.003	1	0	1
320			min	-26.611	3	0	1	.144	15	0	1	0	15	0	1
321		9	max	478.695	1	0	1	3.947	1	0	1	.003	1	0	1
322			min	-26.562	3	0	1	.144	15	0	1	0	15	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	478.76	1	0	1	3.947	1	0	1	.003	1	0	1
324		min	-26.514	3	0	1	.144	15	0	1	0	15	0	1
325	11	max	478.824	1	0	1	3.947	1	0	1	.004	1	0	1
326		min	-26.465	3	0	1	.144	15	0	1	0	15	0	1
327	12	max	478.889	1	0	1	3.947	1	0	1	.004	1	0	1
328		min	-26.417	3	0	1	.144	15	0	1	0	15	0	1
329	13	max	478.954	1	0	1	3.947	1	0	1	.004	1	0	1
330		min	-26.368	3	0	1	.144	15	0	1	0	15	0	1
331	14	max	479.018	1	0	1	3.947	1	0	1	.005	1	0	1
332		min	-26.319	3	0	1	.144	15	0	1	0	15	0	1
333	15	max	479.083	1	0	1	3.947	1	0	1	.005	1	0	1
334		min	-26.271	3	0	1	.144	15	0	1	0	15	0	1
335	16	max	479.148	1	0	1	3.947	1	0	1	.005	1	0	1
336		min	-26.222	3	0	1	.144	15	0	1	0	15	0	1
337	17	max	479.213	1	0	1	3.947	1	0	1	.006	1	0	1
338		min	-26.174	3	0	1	.144	15	0	1	0	15	0	1
339	18	max	479.277	1	0	1	3.947	1	0	1	.006	1	0	1
340		min	-26.125	3	0	1	.144	15	0	1	0	15	0	1
341	19	max	479.342	1	0	1	3.947	1	0	1	.006	1	0	1
342		min	-26.077	3	0	1	.144	15	0	1	0	15	0	1
343	M1	1	max	142.908	1	339.859	3	-2.888	15	0	.155	1	0	1
344		min	5.206	15	-329.79	1	-78.462	1	0	3	.006	15	0	3
345	2	max	143.026	1	339.669	3	-2.888	15	0	1	.138	1	.072	1
346		min	5.241	15	-330.043	1	-78.462	1	0	3	.005	15	-.074	3
347	3	max	97.388	1	7.174	9	-2.865	15	0	12	.12	1	.142	1
348		min	-2.767	10	-18.093	3	-78.276	1	0	1	.004	15	-.146	3
349	4	max	97.506	1	6.964	9	-2.865	15	0	12	.103	1	.142	1
350		min	-2.668	10	-18.283	3	-78.276	1	0	1	.004	15	-.142	3
351	5	max	97.625	1	6.753	9	-2.865	15	0	12	.086	1	.143	1
352		min	-2.57	10	-18.473	3	-78.276	1	0	1	.003	15	-.138	3
353	6	max	97.743	1	6.542	9	-2.865	15	0	12	.069	1	.143	1
354		min	-2.472	10	-18.662	3	-78.276	1	0	1	.003	15	-.134	3
355	7	max	97.861	1	6.331	9	-2.865	15	0	12	.052	1	.143	1
356		min	-2.373	10	-18.89	2	-78.276	1	0	1	.002	15	-.13	3
357	8	max	97.979	1	6.12	9	-2.865	15	0	12	.035	1	.144	1
358		min	-2.275	10	-19.144	2	-78.276	1	0	1	.001	15	-.126	3
359	9	max	98.097	1	5.909	9	-2.865	15	0	12	.018	1	.144	1
360		min	-2.177	10	-19.397	2	-78.276	1	0	1	0	15	-.122	3
361	10	max	98.215	1	5.698	9	-2.865	15	0	12	0	3	.146	2
362		min	-2.078	10	-19.65	2	-78.276	1	0	1	0	10	-.118	3
363	11	max	98.333	1	5.487	9	-2.865	15	0	12	0	12	.15	2
364		min	-1.98	10	-19.903	2	-78.276	1	0	1	-.016	1	-.113	3
365	12	max	98.451	1	5.276	9	-2.865	15	0	12	-.001	12	.154	2
366		min	-1.882	10	-20.156	2	-78.276	1	0	1	-.033	1	-.109	3
367	13	max	98.569	1	5.065	9	-2.865	15	0	12	-.002	12	.159	2
368		min	-1.783	10	-20.409	2	-78.276	1	0	1	-.05	1	-.105	3
369	14	max	98.687	1	4.855	9	-2.865	15	0	12	-.002	15	.163	2
370		min	-1.685	10	-20.662	2	-78.276	1	0	1	-.067	1	-.1	3
371	15	max	98.805	1	4.644	9	-2.865	15	0	12	-.003	15	.168	2
372		min	-1.587	10	-20.915	2	-78.276	1	0	1	-.084	1	-.096	3
373	16	max	88.132	2	60.01	2	-2.889	15	0	1	-.004	15	.172	2
374		min	-19.725	3	-122.464	3	-78.849	1	0	12	-.102	1	-.091	3
375	17	max	88.25	2	59.757	2	-2.889	15	0	1	-.004	15	.162	1
376		min	-19.636	3	-122.654	3	-78.849	1	0	12	-.119	1	-.064	3
377	18	max	-5.219	15	377.089	1	-2.96	15	0	3	-.005	15	.082	1
378		min	-142.525	1	-147.738	3	-80.836	1	0	1	-.136	1	-.032	3
379	19	max	-5.184	15	376.836	1	-2.96	15	0	3	-.006	15	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-142.407	1	-147.928	3	-80.836	1	0	1	-.154	1	0	3
381	M5	max	311.542	1	1125.53	3	-.067	10	0	1	.004	1	0	3
382		min	9.706	12	-1093.621	1	-34.981	3	0	3	0	10	0	1
383		max	311.66	1	1125.341	3	-.067	10	0	1	0	2	.237	1
384		min	9.765	12	-1093.874	1	-34.981	3	0	3	-.004	3	-.244	3
385		max	181.933	3	7.375	9	4.05	3	0	3	0	2	.47	1
386		min	-23.057	10	-70.898	2	-.41	2	0	1	-.011	3	-.483	3
387		max	182.021	3	7.165	9	4.05	3	0	3	0	2	.475	1
388		min	-22.958	10	-71.151	2	-.41	2	0	1	-.01	3	-.469	3
389		max	182.11	3	6.954	9	4.05	3	0	3	0	2	.481	1
390		min	-22.86	10	-71.405	2	-.41	2	0	1	-.009	3	-.454	3
391		max	182.198	3	6.743	9	4.05	3	0	3	0	2	.487	1
392		min	-22.761	10	-71.658	2	-.41	2	0	1	-.008	3	-.44	3
393		max	182.287	3	6.532	9	4.05	3	0	3	0	2	.493	1
394		min	-22.663	10	-71.911	2	-.41	2	0	1	-.008	3	-.426	3
395		max	182.375	3	6.321	9	4.05	3	0	3	0	2	.499	1
396		min	-22.565	10	-72.164	2	-.41	2	0	1	-.007	3	-.412	3
397		max	182.464	3	6.11	9	4.05	3	0	3	0	2	.506	1
398		min	-22.466	10	-72.417	2	-.41	2	0	1	-.006	3	-.397	3
399		max	182.552	3	5.899	9	4.05	3	0	3	0	10	.512	1
400		min	-22.368	10	-72.67	2	-.41	2	0	1	-.005	3	-.383	3
401		max	182.641	3	5.688	9	4.05	3	0	3	0	10	.518	1
402		min	-22.27	10	-72.923	2	-.41	2	0	1	-.004	3	-.369	3
403		max	182.729	3	5.477	9	4.05	3	0	3	0	10	.528	2
404		min	-22.171	10	-73.176	2	-.41	2	0	1	-.003	3	-.354	3
405		max	182.818	3	5.266	9	4.05	3	0	3	0	10	.543	2
406		min	-22.073	10	-73.429	2	-.41	2	0	1	-.002	3	-.34	3
407		max	182.906	3	5.056	9	4.05	3	0	3	0	10	.559	2
408		min	-21.975	10	-73.682	2	-.41	2	0	1	-.002	1	-.325	3
409		max	182.995	3	4.845	9	4.05	3	0	3	0	15	.575	2
410		min	-21.876	10	-73.935	2	-.41	2	0	1	-.002	1	-.31	3
411		max	306.77	2	300.095	2	4.021	3	0	1	0	12	.588	2
412		min	-65.395	3	-379.768	3	-.43	2	0	15	-.001	1	-.293	3
413		max	306.888	2	299.842	2	4.021	3	0	1	0	3	.534	1
414		min	-65.307	3	-379.958	3	-.43	2	0	15	-.001	1	-.211	3
415		max	-10.429	12	1243.951	1	3.682	3	0	3	.002	3	.269	1
416		min	-312.322	1	-487.323	3	-.102	2	0	1	0	1	-.105	3
417		max	-10.37	12	1243.698	1	3.682	3	0	3	.002	3	0	3
418		min	-312.204	1	-487.513	3	-.102	2	0	1	0	2	0	1
419	M9	max	142.26	1	339.838	3	101.093	1	0	3	-.006	15	0	1
420		min	5.18	15	-329.773	1	3.909	15	0	1	-.154	1	0	3
421		max	142.378	1	339.648	3	101.093	1	0	3	-.003	12	.072	1
422		min	5.216	15	-330.026	1	3.909	15	0	1	-.132	1	-.074	3
423		max	97.336	1	7.151	9	74.174	1	0	1	.004	3	.142	1
424		min	-2.265	10	-18.03	3	1.09	12	0	15	-.109	1	-.146	3
425		max	97.454	1	6.94	9	74.174	1	0	1	.004	3	.142	1
426		min	-2.167	10	-18.22	3	1.09	12	0	15	-.093	1	-.142	3
427		max	97.572	1	6.729	9	74.174	1	0	1	.004	3	.143	1
428		min	-2.068	10	-18.409	3	1.09	12	0	15	-.077	1	-.138	3
429		max	97.69	1	6.518	9	74.174	1	0	1	.005	3	.143	1
430		min	-1.97	10	-18.648	2	1.09	12	0	15	-.061	1	-.134	3
431		max	97.808	1	6.307	9	74.174	1	0	1	.005	3	.143	1
432		min	-1.872	10	-18.901	2	1.09	12	0	15	-.045	1	-.13	3
433		max	97.926	1	6.096	9	74.174	1	0	1	.005	3	.144	1
434		min	-1.773	10	-19.154	2	1.09	12	0	15	-.029	1	-.126	3
435		max	98.044	1	5.885	9	74.174	1	0	1	.006	3	.144	1
436		min	-1.675	10	-19.407	2	1.09	12	0	15	-.013	1	-.122	3





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	2.888	15	-329.274	1	5.206	15	0	1	.006	15	0	3
495	M16	1	max	-1.386	12	377.371	1	-5.174	15	0	3	.152	1	0	1
496			min	-78.554	1	-147.947	3	-142.114	1	0	1	.006	15	0	3
497		2	max	-1.386	12	266.084	1	-3.97	15	0	3	.047	1	.105	3
498			min	-78.554	1	-104.417	3	-108.963	1	0	1	.002	15	-.268	1
499		3	max	-1.386	12	154.797	1	-2.766	15	0	3	0	12	.174	3
500			min	-78.554	1	-60.886	3	-75.813	1	0	1	-.03	1	-.443	1
501		4	max	-1.386	12	43.51	1	-1.562	15	0	3	-.003	15	.207	3
502			min	-78.554	1	-17.355	3	-42.662	1	0	1	-.079	1	-.526	1
503		5	max	-1.386	12	26.175	3	-.358	15	0	3	-.004	15	.203	3
504			min	-78.554	1	-67.777	1	-9.512	1	0	1	-.101	1	-.516	1
505		6	max	-1.386	12	69.706	3	23.639	1	0	3	-.003	15	.163	3
506			min	-78.554	1	-179.064	1	.422	12	0	1	-.095	1	-.413	1
507		7	max	-1.386	12	113.237	3	56.79	1	0	3	-.002	15	.087	3
508			min	-78.554	1	-290.351	1	1.59	12	0	1	-.061	1	-.218	1
509		8	max	-1.386	12	156.767	3	89.94	1	0	3	.001	2	.071	1
510			min	-78.554	1	-401.638	1	2.758	12	0	1	-.002	3	-.026	3
511		9	max	-1.386	12	200.298	3	123.091	1	0	3	.089	1	.452	1
512			min	-78.554	1	-512.925	1	3.926	12	0	1	.001	12	-.175	3
513		10	max	-2.96	15	-14.483	15	156.241	1	0	15	.205	1	.926	1
514			min	-80.576	1	-624.212	1	-7.894	3	0	1	.007	12	-.36	3
515		11	max	-2.96	15	512.925	1	-4.108	12	0	1	.089	1	.452	1
516			min	-80.576	1	-200.298	3	-122.782	1	0	3	.003	12	-.175	3
517		12	max	-2.96	15	401.638	1	-2.94	12	0	1	.001	2	.071	1
518			min	-80.576	1	-156.767	3	-89.631	1	0	3	0	3	-.026	3
519		13	max	-2.96	15	290.351	1	-1.772	12	0	1	-.002	12	.087	3
520			min	-80.576	1	-113.237	3	-56.48	1	0	3	-.061	1	-.218	1
521		14	max	-2.96	15	179.064	1	-.604	12	0	1	-.003	12	.163	3
522			min	-80.576	1	-69.706	3	-23.33	1	0	3	-.094	1	-.413	1
523		15	max	-2.96	15	67.777	1	9.821	1	0	1	-.003	12	.203	3
524			min	-80.576	1	-26.175	3	.368	15	0	3	-.1	1	-.516	1
525		16	max	-2.96	15	17.355	3	42.971	1	0	1	-.002	12	.207	3
526			min	-80.576	1	-43.51	1	1.572	15	0	3	-.078	1	-.526	1
527		17	max	-2.96	15	60.886	3	76.122	1	0	1	0	12	.174	3
528			min	-80.576	1	-154.797	1	2.776	15	0	3	-.028	1	-.443	1
529		18	max	-2.96	15	104.417	3	109.273	1	0	1	.049	1	.105	3
530			min	-80.576	1	-266.084	1	3.98	15	0	3	.002	15	-.268	1
531		19	max	-2.96	15	147.947	3	142.423	1	0	1	.154	1	0	1
532			min	-80.576	1	-377.371	1	5.184	15	0	3	.006	15	0	3
533	M15	1	max	0	2	2.617	4	.034	3	0	1	0	1	0	1
534			min	-41.747	3	0	2	-.036	1	0	3	0	3	0	1
535		2	max	0	2	2.326	4	.034	3	0	1	0	1	0	2
536			min	-41.812	3	0	2	-.036	1	0	3	0	3	-.001	4
537		3	max	0	2	2.035	4	.034	3	0	1	0	1	0	2
538			min	-41.877	3	0	2	-.036	1	0	3	0	3	-.002	4
539		4	max	0	2	1.745	4	.034	3	0	1	0	1	0	2
540			min	-41.942	3	0	2	-.036	1	0	3	0	3	-.003	4
541		5	max	0	2	1.454	4	.034	3	0	1	0	1	0	2
542			min	-42.007	3	0	2	-.036	1	0	3	0	3	-.004	4
543		6	max	0	2	1.163	4	.034	3	0	1	0	1	0	2
544			min	-42.073	3	0	2	-.036	1	0	3	0	3	-.004	4
545		7	max	0	2	.872	4	.034	3	0	1	0	3	0	2
546			min	-42.138	3	0	2	-.036	1	0	3	0	1	-.005	4
547		8	max	0	2	.582	4	.034	3	0	1	0	3	0	2
548			min	-42.203	3	0	2	-.036	1	0	3	0	1	-.005	4
549		9	max	0	2	.291	4	.034	3	0	1	0	3	0	2
550			min	-42.268	3	0	2	-.036	1	0	3	0	1	-.005	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	2	0	1	.034	3	0	1	0	3	0	2
552			min	-42.333	3	0	1	-.036	1	0	3	0	1	-.005	4
553		11	max	0	2	0	2	.034	3	0	1	0	3	0	2
554			min	-42.399	3	-.291	4	-.036	1	0	3	0	1	-.005	4
555		12	max	0	2	0	2	.034	3	0	1	0	3	0	2
556			min	-42.464	3	-.582	4	-.036	1	0	3	0	1	-.005	4
557		13	max	0	2	0	2	.034	3	0	1	0	3	0	2
558			min	-42.529	3	-.872	4	-.036	1	0	3	0	1	-.005	4
559		14	max	0	2	0	2	.034	3	0	1	0	3	0	2
560			min	-42.594	3	-1.163	4	-.036	1	0	3	0	1	-.004	4
561		15	max	0	2	0	2	.034	3	0	1	0	3	0	2
562			min	-42.659	3	-1.454	4	-.036	1	0	3	0	1	-.004	4
563		16	max	0	2	0	2	.034	3	0	1	0	3	0	2
564			min	-42.725	3	-1.745	4	-.036	1	0	3	0	1	-.003	4
565		17	max	0	2	0	2	.034	3	0	1	0	3	0	2
566			min	-42.79	3	-2.035	4	-.036	1	0	3	0	1	-.002	4
567		18	max	0	2	0	2	.034	3	0	1	0	3	0	2
568			min	-42.855	3	-2.326	4	-.036	1	0	3	0	1	-.001	4
569		19	max	0	2	0	2	.034	3	0	1	0	3	0	1
570			min	-42.92	3	-2.617	4	-.036	1	0	3	0	1	0	1
571	M16A	1	max	-.853	10	2.617	4	.022	1	0	3	0	3	0	1
572			min	-42.365	3	.615	15	-.013	3	0	1	0	1	0	1
573		2	max	-.78	10	2.326	4	.022	1	0	3	0	3	0	15
574			min	-42.299	3	.547	15	-.013	3	0	1	0	1	-.001	4
575		3	max	-.708	10	2.035	4	.022	1	0	3	0	3	0	15
576			min	-42.234	3	.478	15	-.013	3	0	1	0	1	-.002	4
577		4	max	-.636	10	1.745	4	.022	1	0	3	0	3	0	15
578			min	-42.169	3	.41	15	-.013	3	0	1	0	1	-.003	4
579		5	max	-.563	10	1.454	4	.022	1	0	3	0	3	0	15
580			min	-42.104	3	.342	15	-.013	3	0	1	0	1	-.004	4
581		6	max	-.491	10	1.163	4	.022	1	0	3	0	3	0	15
582			min	-42.039	3	.273	15	-.013	3	0	1	0	1	-.004	4
583		7	max	-.418	10	.872	4	.022	1	0	3	0	3	-.001	15
584			min	-41.973	3	.205	15	-.013	3	0	1	0	1	-.005	4
585		8	max	-.346	10	.582	4	.022	1	0	3	0	3	-.001	15
586			min	-41.908	3	.137	15	-.013	3	0	1	0	1	-.005	4
587		9	max	-.273	10	.291	4	.022	1	0	3	0	3	-.001	15
588			min	-41.843	3	.068	15	-.013	3	0	1	0	1	-.005	4
589		10	max	-.201	10	0	1	.022	1	0	3	0	3	-.001	15
590			min	-41.778	3	0	1	-.013	3	0	1	0	1	-.005	4
591		11	max	-.129	10	-.068	15	.022	1	0	3	0	3	-.001	15
592			min	-41.713	3	-.291	4	-.013	3	0	1	0	1	-.005	4
593		12	max	-.056	10	-.137	15	.022	1	0	3	0	3	-.001	15
594			min	-41.648	3	-.582	4	-.013	3	0	1	0	1	-.005	4
595		13	max	.016	10	-.205	15	.022	1	0	3	0	2	-.001	15
596			min	-41.582	3	-.872	4	-.013	3	0	1	0	3	-.005	4
597		14	max	.089	10	-.273	15	.022	1	0	3	0	1	0	15
598			min	-41.517	3	-1.163	4	-.013	3	0	1	0	3	-.004	4
599		15	max	.161	10	-.342	15	.022	1	0	3	0	1	0	15
600			min	-41.452	3	-1.454	4	-.013	3	0	1	0	3	-.004	4
601		16	max	.234	10	-.41	15	.022	1	0	3	0	1	0	15
602			min	-41.387	3	-1.745	4	-.013	3	0	1	0	3	-.003	4
603		17	max	.306	10	-.478	15	.022	1	0	3	0	1	0	15
604			min	-41.322	3	-2.035	4	-.013	3	0	1	0	3	-.002	4
605		18	max	.378	10	-.547	15	.022	1	0	3	0	1	0	15
606			min	-41.256	3	-2.326	4	-.013	3	0	1	0	3	-.001	4
607		19	max	.451	10	-.615	15	.022	1	0	3	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-41.191	3	-2.617	4	-.013	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.008	2	.015	1	-4.494e-5	15	NC	3	NC	3	
2			min	-.003	3	-.008	3	0	3	-1.224e-3	1	4569.43	2	2442.744	1	
3			2	max	.003	1	.007	2	.014	1	-4.303e-5	15	NC	3	NC	3
4				min	-.003	3	-.007	3	0	3	-1.172e-3	1	4976.945	2	2637.177	1
5			3	max	.003	1	.007	2	.013	1	-4.112e-5	15	NC	3	NC	3
6				min	-.003	3	-.007	3	0	3	-1.121e-3	1	5459.85	2	2866.512	1
7			4	max	.002	1	.006	2	.012	1	-3.92e-5	15	NC	1	NC	3
8				min	-.003	3	-.007	3	0	3	-1.069e-3	1	6036.081	2	3139.155	1
9			5	max	.002	1	.005	2	.01	1	-3.729e-5	15	NC	1	NC	3
10				min	-.003	3	-.006	3	0	3	-1.017e-3	1	6729.476	2	3466.33	1
11		6	max	.002	1	.005	2	.009	1	-3.538e-5	15	NC	1	NC	3	
12			min	-.002	3	-.006	3	0	3	-9.656e-4	1	7572.227	2	3863.291	1	
13		7	max	.002	1	.004	2	.008	1	-3.346e-5	15	NC	1	NC	2	
14			min	-.002	3	-.006	3	0	3	-9.139e-4	1	8608.649	2	4351.204	1	
15		8	max	.002	1	.004	2	.007	1	-3.155e-5	15	NC	1	NC	2	
16			min	-.002	3	-.005	3	0	3	-8.622e-4	1	9901.104	2	4960.144	1	
17		9	max	.002	1	.003	2	.006	1	-2.963e-5	15	NC	1	NC	2	
18			min	-.002	3	-.005	3	0	3	-8.105e-4	1	NC	1	5734.056	1	
19		10	max	.001	1	.003	2	.005	1	-2.772e-5	15	NC	1	NC	2	
20			min	-.002	3	-.005	3	0	3	-7.588e-4	1	NC	1	6739.302	1	
21		11	max	.001	1	.002	2	.004	1	-2.581e-5	15	NC	1	NC	2	
22			min	-.001	3	-.004	3	0	3	-7.071e-4	1	NC	1	8080.139	1	
23		12	max	.001	1	.002	2	.004	1	-2.389e-5	15	NC	1	NC	2	
24			min	-.001	3	-.004	3	0	3	-6.553e-4	1	NC	1	9928.461	1	
25		13	max	0	1	.001	2	.003	1	-2.198e-5	15	NC	1	NC	1	
26			min	-.001	3	-.003	3	0	3	-6.036e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-2.007e-5	15	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-5.519e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-1.815e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-5.002e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-1.624e-5	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-4.485e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.432e-5	15	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-3.968e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.241e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.451e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-8.664e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.934e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.366e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	4.154e-6	12	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	1.701e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	6.13e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	2.036e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	7.372e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	2.371e-4	1	NC	1	NC	1
46				min	0	2	-.002	3	-.001	1	8.614e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	12	2.707e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	-.001	1	9.857e-6	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	3.042e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.11e-5	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	3.377e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.005	3	0	1	1.234e-5	15	NC	1	NC	1
53	8	max	0	3	0	2	0	3	3.712e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	1.358e-5	15	NC	1	NC	1
55	9	max	0	3	.001	2	0	3	4.048e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	2	1.483e-5	15	NC	1	NC	1
57	10	max	0	3	.002	2	0	1	4.383e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	15	1.607e-5	15	NC	1	NC	1
59	11	max	0	3	.002	2	.001	1	4.718e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	15	1.731e-5	15	NC	1	NC	1
61	12	max	0	3	.003	2	.002	1	5.054e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	1.855e-5	15	NC	1	NC	1
63	13	max	0	3	.003	2	.003	1	5.389e-4	1	NC	1	NC	1
64		min	0	2	-.007	3	0	15	1.98e-5	15	NC	1	NC	1
65	14	max	.001	3	.004	2	.004	1	5.724e-4	1	NC	1	NC	1
66		min	0	2	-.007	3	0	15	2.104e-5	15	NC	1	NC	1
67	15	max	.001	3	.005	2	.004	1	6.059e-4	1	NC	1	NC	1
68		min	0	2	-.007	3	0	15	2.228e-5	15	9338.298	2	NC	1
69	16	max	.001	3	.006	2	.005	1	6.395e-4	1	NC	1	NC	2
70		min	0	2	-.008	3	0	15	2.352e-5	15	7891.463	2	8630.278	1
71	17	max	.001	3	.007	2	.006	1	6.73e-4	1	NC	3	NC	2
72		min	-.001	2	-.008	3	0	15	2.476e-5	15	6776.051	2	7428.196	1
73	18	max	.001	3	.008	2	.007	1	7.065e-4	1	NC	3	NC	2
74		min	-.001	2	-.008	3	0	15	2.601e-5	15	5906.032	2	6537.428	1
75	19	max	.001	3	.009	2	.008	1	7.401e-4	1	NC	3	NC	2
76		min	-.001	2	-.008	3	0	15	2.725e-5	15	5221.162	2	5862.164	1
77	M4	1	max	.002	1	.009	2	15	-3.536e-5	15	NC	1	NC	3
78		min	0	3	-.008	3	-.006	1	-9.749e-4	1	NC	1	3259.909	1
79	2	max	.002	1	.009	2	0	15	-3.536e-5	15	NC	1	NC	3
80		min	0	3	-.007	3	-.005	1	-9.749e-4	1	NC	1	3556.611	1
81	3	max	.002	1	.008	2	0	15	-3.536e-5	15	NC	1	NC	2
82		min	0	3	-.007	3	-.005	1	-9.749e-4	1	NC	1	3909.719	1
83	4	max	.002	1	.008	2	0	15	-3.536e-5	15	NC	1	NC	2
84		min	0	3	-.006	3	-.004	1	-9.749e-4	1	NC	1	4334.111	1
85	5	max	.002	1	.007	2	0	15	-3.536e-5	15	NC	1	NC	2
86		min	0	3	-.006	3	-.004	1	-9.749e-4	1	NC	1	4850.061	1
87	6	max	.002	1	.007	2	0	15	-3.536e-5	15	NC	1	NC	2
88		min	0	3	-.005	3	-.004	1	-9.749e-4	1	NC	1	5485.766	1
89	7	max	.002	1	.006	2	0	15	-3.536e-5	15	NC	1	NC	2
90		min	0	3	-.005	3	-.003	1	-9.749e-4	1	NC	1	6281.389	1
91	8	max	.001	1	.006	2	0	15	-3.536e-5	15	NC	1	NC	2
92		min	0	3	-.005	3	-.003	1	-9.749e-4	1	NC	1	7295.751	1
93	9	max	.001	1	.005	2	0	15	-3.536e-5	15	NC	1	NC	2
94		min	0	3	-.004	3	-.002	1	-9.749e-4	1	NC	1	8617.827	1
95	10	max	.001	1	.005	2	0	15	-3.536e-5	15	NC	1	NC	1
96		min	0	3	-.004	3	-.002	1	-9.749e-4	1	NC	1	NC	1
97	11	max	.001	1	.004	2	0	15	-3.536e-5	15	NC	1	NC	1
98		min	0	3	-.003	3	-.002	1	-9.749e-4	1	NC	1	NC	1
99	12	max	0	1	.004	2	0	15	-3.536e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	-.001	1	-9.749e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-3.536e-5	15	NC	1	NC	1
102		min	0	3	-.003	3	0	1	-9.749e-4	1	NC	1	NC	1
103	14	max	0	1	.003	2	0	15	-3.536e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-9.749e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-3.536e-5	15	NC	1	NC	1
106		min	0	3	-.002	3	0	1	-9.749e-4	1	NC	1	NC	1
107	16	max	0	1	.002	2	0	15	-3.536e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-9.749e-4	1	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-3.536e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-9.749e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.536e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-9.749e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.536e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.749e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.029	2	.005	1	2.54e-4	3	NC	3	NC	2
116			min	-.011	3	-.024	3	-.003	3	2.138e-6	10	1238.073	2	7826.214	1
117		2	max	.009	1	.027	2	.004	1	2.464e-4	3	NC	3	NC	2
118			min	-.01	3	-.023	3	-.003	3	1.417e-6	10	1322.718	2	8491.276	1
119		3	max	.009	1	.026	2	.004	1	2.388e-4	3	NC	3	NC	2
120			min	-.009	3	-.022	3	-.003	3	6.95e-7	10	1419.449	2	9279.462	1
121		4	max	.008	1	.024	2	.004	1	2.312e-4	3	NC	3	NC	1
122			min	-.009	3	-.02	3	-.003	3	0	10	1530.681	2	NC	1
123		5	max	.008	1	.022	2	.003	1	2.236e-4	3	NC	3	NC	1
124			min	-.008	3	-.019	3	-.002	3	-7.483e-7	10	1659.524	2	NC	1
125		6	max	.007	1	.02	2	.003	1	2.159e-4	3	NC	3	NC	1
126			min	-.008	3	-.018	3	-.002	3	-2.437e-6	2	1810.049	2	NC	1
127		7	max	.006	1	.018	2	.003	1	2.083e-4	3	NC	3	NC	1
128			min	-.007	3	-.017	3	-.002	3	-5.897e-6	2	1987.688	2	NC	1
129		8	max	.006	1	.017	2	.002	1	2.007e-4	3	NC	3	NC	1
130			min	-.006	3	-.015	3	-.002	3	-9.356e-6	2	2199.851	2	NC	1
131		9	max	.005	1	.015	2	.002	1	1.931e-4	3	NC	3	NC	1
132			min	-.006	3	-.014	3	-.001	3	-1.282e-5	2	2456.914	2	NC	1
133		10	max	.005	1	.013	2	.002	1	1.854e-4	3	NC	3	NC	1
134			min	-.005	3	-.013	3	-.001	3	-1.628e-5	2	2773.865	2	NC	1
135		11	max	.004	1	.011	2	.001	1	1.778e-4	3	NC	3	NC	1
136			min	-.005	3	-.011	3	-.001	3	-1.974e-5	2	3173.186	2	NC	1
137		12	max	.004	1	.01	2	.001	1	1.702e-4	3	NC	3	NC	1
138			min	-.004	3	-.01	3	0	3	-2.32e-5	2	3690.201	2	NC	1
139		13	max	.003	1	.008	2	0	1	1.626e-4	3	NC	3	NC	1
140			min	-.004	3	-.009	3	0	3	-2.666e-5	2	4383.778	2	NC	1
141		14	max	.003	1	.007	2	0	1	1.55e-4	3	NC	3	NC	1
142			min	-.003	3	-.007	3	0	3	-3.011e-5	2	5359.864	2	NC	1
143		15	max	.002	1	.005	2	0	1	1.473e-4	3	NC	3	NC	1
144			min	-.002	3	-.006	3	0	3	-3.357e-5	2	6830.333	2	NC	1
145		16	max	.002	1	.004	2	0	1	1.397e-4	3	NC	1	NC	1
146			min	-.002	3	-.004	3	0	3	-3.703e-5	2	9289.519	2	NC	1
147		17	max	.001	1	.003	2	0	1	1.321e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-4.049e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.245e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-4.395e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.169e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.741e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.184e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-5.412e-5	3	NC	1	NC	1
155		2	max	0	3	.002	2	0	3	1.893e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-4.e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.805e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-2.587e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.717e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.175e-5	3	NC	1	NC	1
161		5	max	.001	3	.006	2	0	3	1.629e-5	1	NC	3	NC	1
162			min	-.001	2	-.007	3	0	1	4.616e-7	15	7915.461	2	NC	1
163		6	max	.001	3	.007	2	.001	3	1.649e-5	3	NC	3	NC	1
164			min	-.001	2	-.009	3	0	1	5.16e-7	15	6347.85	2	NC	1
165		7	max	.002	3	.009	2	.001	3	3.061e-5	3	NC	3	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.002	2	-.011	3	0	1	-6.413e-8	2	5277.881	2	NC	1
167		8	max	.002	3	.01	2	.001	3	4.474e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-3.715e-6	2	4494.862	2	NC	1
169		9	max	.002	3	.012	2	.002	3	5.886e-5	3	NC	3	NC	1
170			min	-.002	2	-.014	3	0	1	-7.366e-6	2	3893.779	2	NC	1
171		10	max	.002	3	.013	2	.002	3	7.298e-5	3	NC	3	NC	1
172			min	-.003	2	-.015	3	-.001	1	-1.102e-5	2	3416.428	2	NC	1
173		11	max	.003	3	.015	2	.002	3	8.71e-5	3	NC	3	NC	1
174			min	-.003	2	-.016	3	-.001	1	-1.467e-5	2	3027.879	2	NC	1
175		12	max	.003	3	.017	2	.002	3	1.012e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.001	1	-1.832e-5	2	2705.811	2	NC	1
177		13	max	.003	3	.019	2	.002	3	1.153e-4	3	NC	3	NC	1
178			min	-.004	2	-.019	3	-.001	1	-2.197e-5	2	2435.202	2	NC	1
179		14	max	.003	3	.021	2	.002	3	1.295e-4	3	NC	3	NC	1
180			min	-.004	2	-.02	3	-.002	1	-2.562e-5	2	2205.495	2	NC	1
181		15	max	.004	3	.023	2	.002	3	1.436e-4	3	NC	3	NC	1
182			min	-.004	2	-.021	3	-.002	1	-2.927e-5	2	2008.998	2	NC	1
183		16	max	.004	3	.025	2	.002	3	1.577e-4	3	NC	3	NC	1
184			min	-.004	2	-.022	3	-.002	1	-3.292e-5	2	1839.926	2	NC	1
185		17	max	.004	3	.027	2	.002	3	1.718e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	-.002	1	-3.657e-5	2	1693.827	2	NC	1
187		18	max	.004	3	.029	2	.002	3	1.86e-4	3	NC	3	NC	1
188			min	-.005	2	-.023	3	-.002	1	-4.022e-5	2	1567.199	2	NC	1
189		19	max	.005	3	.032	2	.002	3	2.001e-4	3	NC	3	NC	1
190			min	-.005	2	-.024	3	-.002	1	-4.388e-5	2	1457.25	2	NC	1
191	M8	1	max	.006	1	.033	2	.002	1	-2.39e-6	10	NC	1	NC	2
192			min	0	3	-.024	3	-.001	3	-1.576e-4	3	NC	1	7999.19	1
193		2	max	.006	1	.031	2	.002	1	-2.39e-6	10	NC	1	NC	2
194			min	0	3	-.023	3	0	3	-1.576e-4	3	NC	1	8721.282	1
195		3	max	.006	1	.03	2	.002	1	-2.39e-6	10	NC	1	NC	2
196			min	0	3	-.022	3	0	3	-1.576e-4	3	NC	1	9580.933	1
197		4	max	.005	1	.028	2	.002	1	-2.39e-6	10	NC	1	NC	1
198			min	0	3	-.02	3	0	3	-1.576e-4	3	NC	1	NC	1
199		5	max	.005	1	.026	2	.002	1	-2.39e-6	10	NC	1	NC	1
200			min	0	3	-.019	3	0	3	-1.576e-4	3	NC	1	NC	1
201		6	max	.004	1	.024	2	.001	1	-2.39e-6	10	NC	1	NC	1
202			min	0	3	-.017	3	0	3	-1.576e-4	3	NC	1	NC	1
203		7	max	.004	1	.022	2	.001	1	-2.39e-6	10	NC	1	NC	1
204			min	0	3	-.016	3	0	3	-1.576e-4	3	NC	1	NC	1
205		8	max	.004	1	.02	2	.001	1	-2.39e-6	10	NC	1	NC	1
206			min	0	3	-.015	3	0	3	-1.576e-4	3	NC	1	NC	1
207		9	max	.003	1	.019	2	0	1	-2.39e-6	10	NC	1	NC	1
208			min	0	3	-.013	3	0	3	-1.576e-4	3	NC	1	NC	1
209		10	max	.003	1	.017	2	0	1	-2.39e-6	10	NC	1	NC	1
210			min	0	3	-.012	3	0	3	-1.576e-4	3	NC	1	NC	1
211		11	max	.003	1	.015	2	0	1	-2.39e-6	10	NC	1	NC	1
212			min	0	3	-.011	3	0	3	-1.576e-4	3	NC	1	NC	1
213		12	max	.002	1	.013	2	0	1	-2.39e-6	10	NC	1	NC	1
214			min	0	3	-.009	3	0	3	-1.576e-4	3	NC	1	NC	1
215		13	max	.002	1	.011	2	0	1	-2.39e-6	10	NC	1	NC	1
216			min	0	3	-.008	3	0	3	-1.576e-4	3	NC	1	NC	1
217		14	max	.002	1	.009	2	0	1	-2.39e-6	10	NC	1	NC	1
218			min	0	3	-.007	3	0	3	-1.576e-4	3	NC	1	NC	1
219		15	max	.001	1	.007	2	0	1	-2.39e-6	10	NC	1	NC	1
220			min	0	3	-.005	3	0	3	-1.576e-4	3	NC	1	NC	1
221		16	max	.001	1	.006	2	0	1	-2.39e-6	10	NC	1	NC	1
222			min	0	3	-.004	3	0	3	-1.576e-4	3	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	1	-2.39e-6	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-1.576e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-2.39e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.576e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-2.39e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.576e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.008	2	0	3	1.052e-3	1	NC	3	NC	1
230			min	-.003	3	-.008	3	-.002	1	-2.541e-4	3	4573.492	2	NC	1
231		2	max	.003	1	.007	2	0	3	9.978e-4	1	NC	3	NC	1
232			min	-.003	3	-.007	3	-.002	1	-2.465e-4	3	4981.493	2	NC	1
233		3	max	.003	1	.007	2	0	3	9.436e-4	1	NC	3	NC	1
234			min	-.003	3	-.007	3	-.002	1	-2.389e-4	3	5464.997	2	NC	1
235		4	max	.003	1	.006	2	0	3	8.894e-4	1	NC	1	NC	1
236			min	-.003	3	-.007	3	-.002	1	-2.313e-4	3	6041.974	2	NC	1
237		5	max	.002	1	.005	2	0	3	8.352e-4	1	NC	1	NC	1
238			min	-.002	3	-.006	3	-.002	1	-2.237e-4	3	6736.305	2	NC	1
239		6	max	.002	1	.005	2	0	3	7.81e-4	1	NC	1	NC	1
240			min	-.002	3	-.006	3	-.002	1	-2.161e-4	3	7580.248	2	NC	1
241		7	max	.002	1	.004	2	0	3	7.268e-4	1	NC	1	NC	1
242			min	-.002	3	-.006	3	-.001	1	-2.085e-4	3	8618.206	2	NC	1
243		8	max	.002	1	.004	2	0	3	6.726e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	-.001	1	-2.009e-4	3	9912.68	2	NC	1
245		9	max	.002	1	.003	2	0	3	6.184e-4	1	NC	1	NC	1
246			min	-.002	3	-.005	3	-.001	1	-1.933e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	5.642e-4	1	NC	1	NC	1
248			min	-.002	3	-.005	3	0	1	-1.857e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	5.1e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.782e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.558e-4	1	NC	1	NC	1
252			min	-.001	3	-.004	3	0	1	-1.706e-4	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	4.017e-4	1	NC	1	NC	1
254			min	-.001	3	-.003	3	0	1	-1.63e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.475e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-1.554e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.933e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.478e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.391e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.402e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.849e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-1.326e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.307e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.25e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	7.649e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.174e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	5.464e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.694e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	3.885e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-9.831e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	2	2.306e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.597e-4	1	NC	1	NC	1
273		4	max	0	3	0	2	0	10	7.268e-6	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-2.21e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	-5.986e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	0	3	-2.824e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-1.251e-5	15	NC	1	NC	1
278			min	0	2	-.004	3	-.001	1	-3.438e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	15	-1.483e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	1	-4.051e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	15	-1.714e-5	15	NC	1	NC	1
282			min	0	2	-.005	3	-.003	1	-4.665e-4	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	15	-1.946e-5	15	NC	1	NC	1
284			min	0	2	-.006	3	-.004	1	-5.279e-4	1	NC	1	NC	1
285		10	max	0	3	.002	2	0	15	-2.178e-5	15	NC	1	NC	2
286			min	0	2	-.006	3	-.005	1	-5.892e-4	1	NC	1	9106.767	1
287		11	max	0	3	.002	2	0	15	-2.409e-5	15	NC	1	NC	2
288			min	0	2	-.007	3	-.006	1	-6.506e-4	1	NC	1	7474.524	1
289		12	max	0	3	.003	2	0	15	-2.641e-5	15	NC	1	NC	2
290			min	0	2	-.007	3	-.007	1	-7.119e-4	1	NC	1	6297.736	1
291		13	max	0	3	.003	2	0	15	-2.873e-5	15	NC	1	NC	2
292			min	0	2	-.007	3	-.008	1	-7.733e-4	1	NC	1	5421.744	1
293		14	max	.001	3	.004	2	0	15	-3.104e-5	15	NC	1	NC	2
294			min	0	2	-.007	3	-.01	1	-8.347e-4	1	NC	1	4753.091	1
295		15	max	.001	3	.005	2	0	15	-3.336e-5	15	NC	1	NC	2
296			min	0	2	-.008	3	-.011	1	-8.96e-4	1	9350.766	2	4232.559	1
297		16	max	.001	3	.006	2	0	15	-3.568e-5	15	NC	1	NC	2
298			min	0	2	-.008	3	-.012	1	-9.574e-4	1	7900.989	2	3821.154	1
299		17	max	.001	3	.007	2	0	15	-3.799e-5	15	NC	3	NC	3
300			min	-.001	2	-.008	3	-.013	1	-1.019e-3	1	6783.532	2	3492.385	1
301		18	max	.001	3	.008	2	0	15	-4.031e-5	15	NC	3	NC	3
302			min	-.001	2	-.008	3	-.014	1	-1.08e-3	1	5912.063	2	3227.804	1
303		19	max	.001	3	.009	2	0	15	-4.263e-5	15	NC	3	NC	3
304			min	-.001	2	-.008	3	-.015	1	-1.141e-3	1	5226.147	2	3014.323	1
305	M12	1	max	.002	1	.009	2	.013	1	1.028e-3	1	NC	1	NC	3
306			min	0	3	-.008	3	0	15	3.902e-5	15	NC	1	1532.841	1
307		2	max	.002	1	.009	2	.012	1	1.028e-3	1	NC	1	NC	3
308			min	0	3	-.007	3	0	15	3.902e-5	15	NC	1	1671.763	1
309		3	max	.002	1	.008	2	.011	1	1.028e-3	1	NC	1	NC	3
310			min	0	3	-.007	3	0	15	3.902e-5	15	NC	1	1837.123	1
311		4	max	.002	1	.008	2	.009	1	1.028e-3	1	NC	1	NC	3
312			min	0	3	-.006	3	0	15	3.902e-5	15	NC	1	2035.89	1
313		5	max	.002	1	.007	2	.008	1	1.028e-3	1	NC	1	NC	3
314			min	0	3	-.006	3	0	15	3.902e-5	15	NC	1	2277.559	1
315		6	max	.002	1	.007	2	.008	1	1.028e-3	1	NC	1	NC	3
316			min	0	3	-.005	3	0	15	3.902e-5	15	NC	1	2575.336	1
317		7	max	.002	1	.006	2	.007	1	1.028e-3	1	NC	1	NC	3
318			min	0	3	-.005	3	0	15	3.902e-5	15	NC	1	2948.031	1
319		8	max	.001	1	.006	2	.006	1	1.028e-3	1	NC	1	NC	3
320			min	0	3	-.005	3	0	15	3.902e-5	15	NC	1	3423.191	1
321		9	max	.001	1	.005	2	.005	1	1.028e-3	1	NC	1	NC	2
322			min	0	3	-.004	3	0	15	3.902e-5	15	NC	1	4042.483	1
323		10	max	.001	1	.005	2	.004	1	1.028e-3	1	NC	1	NC	2
324			min	0	3	-.004	3	0	15	3.902e-5	15	NC	1	4871.42	1
325		11	max	.001	1	.004	2	.003	1	1.028e-3	1	NC	1	NC	2
326			min	0	3	-.003	3	0	15	3.902e-5	15	NC	1	6017.909	1
327		12	max	0	1	.004	2	.003	1	1.028e-3	1	NC	1	NC	2
328			min	0	3	-.003	3	0	15	3.902e-5	15	NC	1	7670.197	1
329		13	max	0	1	.003	2	.002	1	1.028e-3	1	NC	1	NC	1
330			min	0	3	-.003	3	0	15	3.902e-5	15	NC	1	NC	1
331		14	max	0	1	.003	2	.001	1	1.028e-3	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	3.902e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	1.028e-3	1	NC	1	NC	1
334			min	0	3	-.002	3	0	15	3.902e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	1.028e-3	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	3.902e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	1.028e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.902e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.028e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.902e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.028e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.902e-5	15	NC	1	NC	1
343	M1	1	max	.007	3	.023	3	.002	3	2.52e-2	1	NC	1	NC	1
344			min	-.008	2	-.024	1	-.005	1	-2.588e-2	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.001	3	1.213e-2	1	NC	4	NC	2
346			min	-.008	2	-.013	1	-.011	1	-1.282e-2	3	4351.718	1	7735.211	1
347		3	max	.007	3	.004	3	0	3	6.808e-6	3	NC	4	NC	2
348			min	-.008	2	-.003	1	-.015	1	-7.032e-4	1	2245.258	1	4691.215	1
349		4	max	.007	3	.005	1	0	3	1.115e-5	3	NC	5	NC	2
350			min	-.008	2	-.003	3	-.017	1	-5.947e-4	1	1587.083	1	3882.524	1
351		5	max	.007	3	.012	1	0	3	1.55e-5	3	NC	5	NC	3
352			min	-.008	2	-.01	3	-.017	1	-4.863e-4	1	1270.841	1	3728.876	1
353		6	max	.007	3	.018	1	0	3	1.985e-5	3	NC	5	NC	2
354			min	-.008	2	-.015	3	-.016	1	-3.778e-4	1	1092.098	1	3991.027	1
355		7	max	.007	3	.023	1	0	3	2.42e-5	3	NC	5	NC	2
356			min	-.008	2	-.019	3	-.014	1	-2.694e-4	1	983.848	1	4754.003	1
357		8	max	.007	3	.026	1	0	3	2.854e-5	3	NC	5	NC	2
358			min	-.008	2	-.021	3	-.012	1	-1.609e-4	1	918.238	1	6530.623	1
359		9	max	.007	3	.028	1	0	3	3.289e-5	3	NC	5	NC	1
360			min	-.008	2	-.023	3	-.008	1	-5.248e-5	1	882.259	1	NC	1
361		10	max	.007	3	.029	1	0	3	5.597e-5	1	NC	5	NC	1
362			min	-.008	2	-.023	3	-.005	1	2.388e-6	15	869.908	1	NC	1
363		11	max	.007	3	.028	2	0	3	1.644e-4	1	NC	5	NC	1
364			min	-.008	2	-.022	3	-.001	1	6.34e-6	15	874.035	2	NC	1
365		12	max	.007	3	.026	2	.002	1	2.729e-4	1	NC	5	NC	2
366			min	-.008	2	-.02	3	0	15	1.029e-5	15	900.254	2	7453.825	1
367		13	max	.007	3	.023	2	.005	1	3.813e-4	1	NC	5	NC	2
368			min	-.008	2	-.017	3	0	15	1.424e-5	15	954.927	2	5190.886	1
369		14	max	.007	3	.018	2	.007	1	4.898e-4	1	NC	5	NC	2
370			min	-.008	2	-.014	3	0	15	1.82e-5	15	1049.908	2	4262.126	1
371		15	max	.007	3	.012	2	.008	1	5.982e-4	1	NC	5	NC	2
372			min	-.008	2	-.009	3	0	15	2.215e-5	15	1211.067	2	3929.102	1
373		16	max	.007	3	.004	1	.008	1	6.757e-4	1	NC	4	NC	2
374			min	-.008	2	-.003	3	0	15	2.499e-5	15	1501.291	2	4052.066	1
375		17	max	.007	3	.003	3	.006	1	2.92e-5	3	NC	4	NC	2
376			min	-.008	2	-.005	2	0	15	1.383e-6	15	2118.668	2	4863.924	1
377		18	max	.007	3	.01	3	.002	1	1.431e-2	1	NC	4	NC	2
378			min	-.008	2	-.016	2	0	15	-5.691e-3	3	4100.219	2	7983.559	1
379		19	max	.007	3	.018	3	0	3	2.888e-2	1	NC	1	NC	1
380			min	-.008	2	-.028	2	-.003	1	-1.152e-2	3	NC	1	NC	1
381	M5	1	max	.022	3	.075	3	.002	3	9.913e-7	3	NC	1	NC	1
382			min	-.028	2	-.083	1	-.006	1	5.172e-8	10	NC	1	NC	1
383		2	max	.022	3	.043	3	.002	3	6.659e-5	3	NC	5	NC	1
384			min	-.028	2	-.047	1	-.005	1	-7.516e-5	1	1263.538	1	NC	1
385		3	max	.022	3	.013	3	.003	3	1.309e-4	3	NC	5	NC	1
386			min	-.028	2	-.012	1	-.005	1	-1.495e-4	1	651.047	1	NC	1
387		4	max	.022	3	.017	1	.004	3	1.281e-4	3	NC	5	NC	1
388			min	-.028	2	-.011	3	-.004	1	-1.409e-4	1	459.018	1	NC	1
389		5	max	.022	3	.042	1	.004	3	1.253e-4	3	NC	5	NC	1
390			min	-.028	2	-.032	3	-.004	1	-1.323e-4	1	366.58	1	NC	1
391		6	max	.022	3	.063	1	.004	3	1.225e-4	3	NC	15	NC	1
392			min	-.028	2	-.048	3	-.003	1	-1.238e-4	1	314.193	1	NC	1
393		7	max	.022	3	.079	1	.004	3	1.197e-4	3	NC	15	NC	1





Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	-0.009	15	.011	1	8.544e-3	3	NC	15	NC	4
566			min	0	10	-.039	4	-.008	3	-1.013e-2	1	2548.099	4	2850.851	1
567		18	max	.001	3	-.005	15	.007	3	9.057e-3	3	NC	5	NC	4
568			min	0	10	-.021	4	-.011	2	-1.076e-2	1	5007.408	4	5078.884	1
569		19	max	.001	3	.003	3	.026	3	9.57e-3	3	NC	1	NC	1
570			min	-.001	10	-.003	9	-.03	2	-1.139e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	3	.008	3	2.851e-3	3	NC	1	NC	1
572			min	-.001	3	-.002	1	-.008	2	-2.895e-3	2	NC	1	NC	1
573		2	max	0	10	-.005	15	.005	1	2.732e-3	3	NC	5	NC	2
574			min	-.001	3	-.02	4	-.001	10	-2.766e-3	2	5007.408	4	9452.216	1
575		3	max	0	10	-.009	15	.013	1	2.613e-3	3	NC	15	NC	4
576			min	0	3	-.039	4	-.004	3	-2.637e-3	2	2548.099	4	5346.673	1
577		4	max	0	10	-.013	15	.019	1	2.494e-3	3	7436.859	15	NC	4
578			min	0	3	-.056	4	-.007	3	-2.509e-3	2	1748.145	4	4065.344	1
579		5	max	0	10	-.017	15	.023	1	2.375e-3	3	5803.057	15	NC	4
580			min	0	3	-.072	4	-.01	3	-2.38e-3	2	1364.095	4	3509.822	1
581		6	max	0	10	-.02	15	.026	1	2.256e-3	3	4883.886	15	NC	4
582			min	0	3	-.085	4	-.012	3	-2.252e-3	2	1148.03	4	3266.882	1
583		7	max	0	10	-.022	15	.027	1	2.137e-3	3	4331.126	15	NC	4
584			min	0	3	-.096	4	-.012	3	-2.123e-3	2	1018.096	4	3207.026	1
585		8	max	0	10	-.024	15	.026	1	2.018e-3	3	3999.386	15	NC	4
586			min	0	3	-.104	4	-.012	3	-1.995e-3	2	940.115	4	3285.961	1
587		9	max	0	10	-.025	15	.025	1	1.899e-3	3	3820.825	15	NC	4
588			min	0	3	-.108	4	-.012	3	-1.866e-3	2	898.142	4	3497.702	1
589		10	max	0	10	-.026	15	.022	1	1.78e-3	3	3764.336	15	NC	4
590			min	0	3	-.11	4	-.011	3	-1.737e-3	2	884.863	4	3863.725	1
591		11	max	0	10	-.025	15	.019	1	1.661e-3	3	3820.825	15	NC	4
592			min	0	3	-.108	4	-.009	3	-1.609e-3	2	898.142	4	4437.688	1
593		12	max	0	10	-.024	15	.016	1	1.542e-3	3	3999.386	15	NC	4
594			min	0	3	-.104	4	-.007	3	-1.48e-3	2	940.115	4	5326.29	1
595		13	max	0	10	-.022	15	.013	1	1.423e-3	3	4331.126	15	NC	3
596			min	0	3	-.096	4	-.006	3	-1.352e-3	2	1018.096	4	6742.872	1
597		14	max	0	10	-.02	15	.009	1	1.304e-3	3	4883.886	15	NC	2
598			min	0	3	-.085	4	-.004	3	-1.223e-3	2	1148.03	4	9150.474	1
599		15	max	0	10	-.017	15	.006	1	1.185e-3	3	5803.057	15	NC	1
600			min	0	3	-.071	4	-.002	3	-1.094e-3	2	1364.095	4	NC	1
601		16	max	0	10	-.013	15	.003	1	1.066e-3	3	7436.859	15	NC	1
602			min	0	3	-.056	4	0	3	-9.659e-4	2	1748.145	4	NC	1
603		17	max	0	10	-.009	15	.001	1	9.472e-4	3	NC	15	NC	1
604			min	0	3	-.038	4	0	10	-8.373e-4	2	2548.099	4	NC	1
605		18	max	0	10	-.005	15	0	3	8.282e-4	3	NC	5	NC	1
606			min	0	3	-.019	4	0	2	-7.087e-4	2	5007.408	4	NC	1
607		19	max	0	1	0	1	0	1	7.092e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-5.802e-4	2	NC	1	NC	1



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

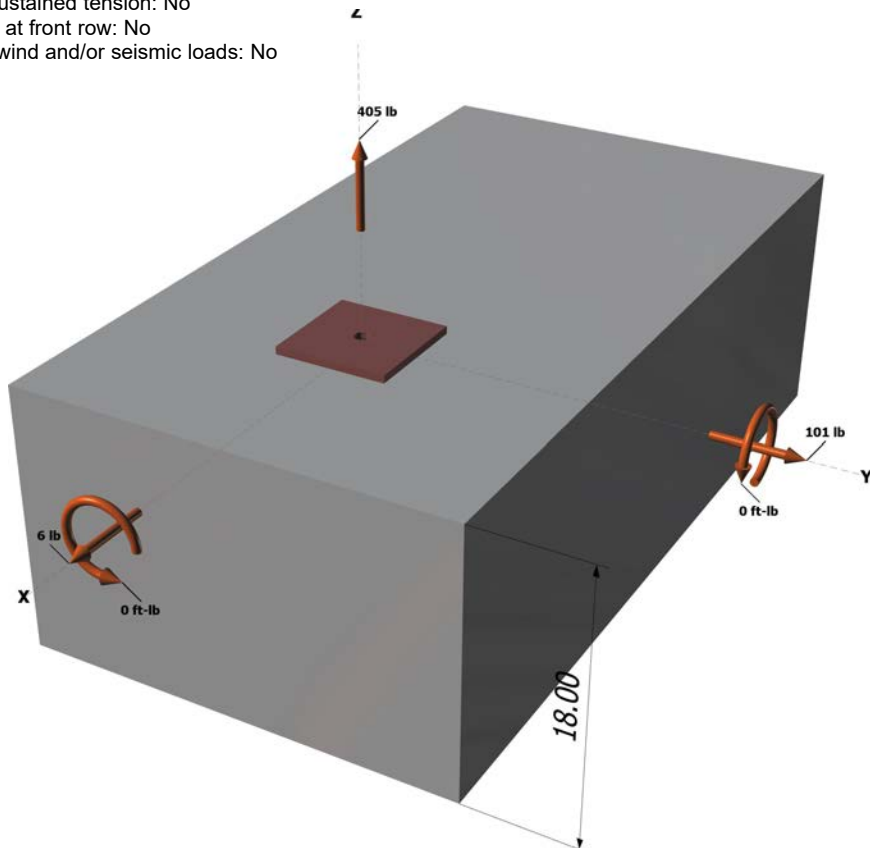
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

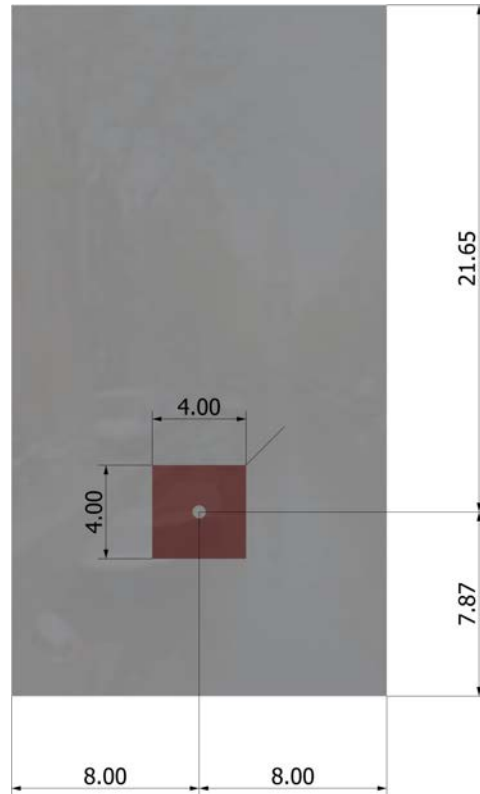
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

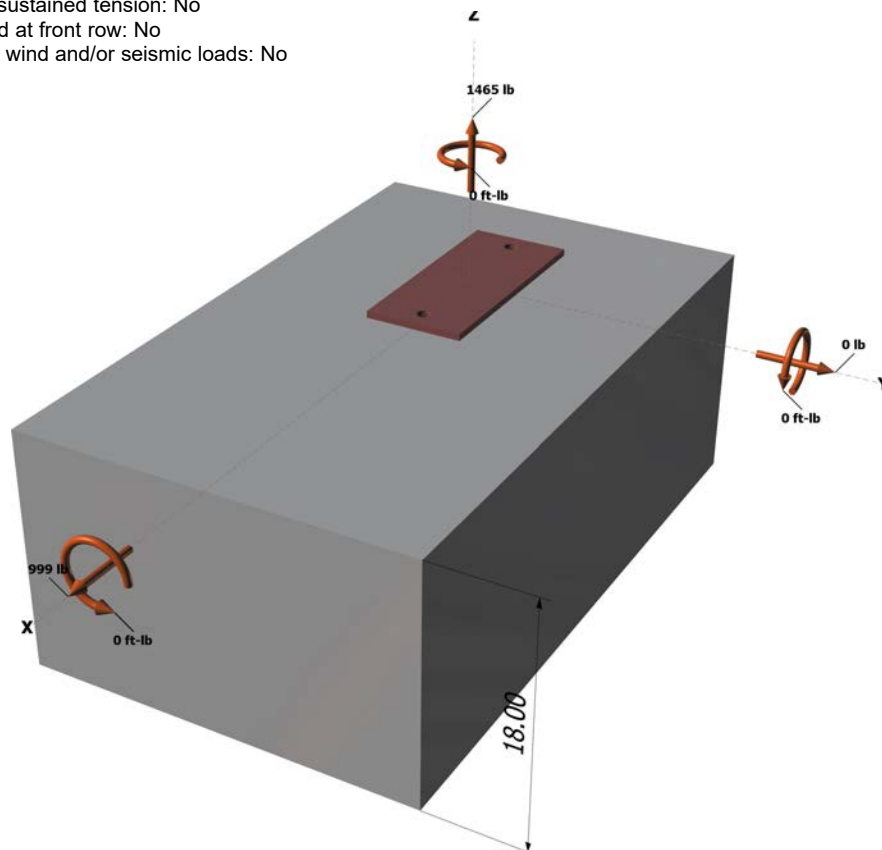
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

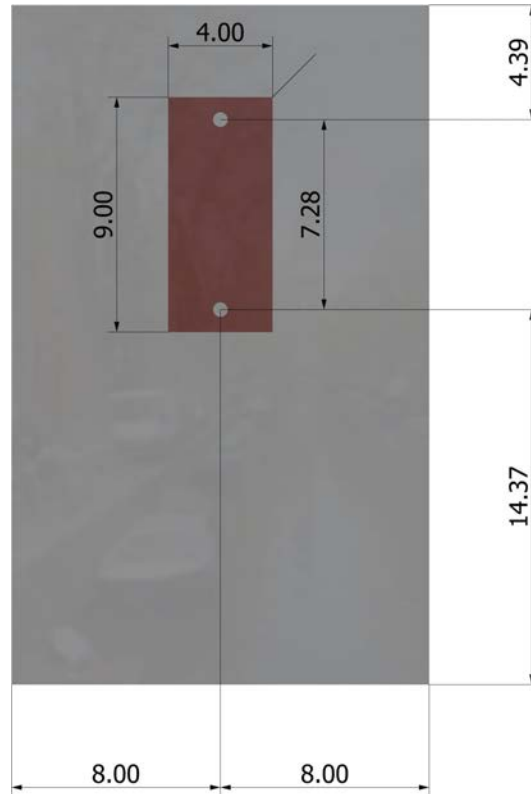
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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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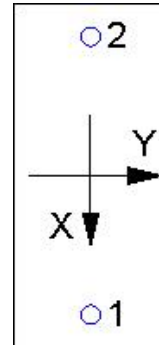
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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1465
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.