



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	115 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 20.76 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

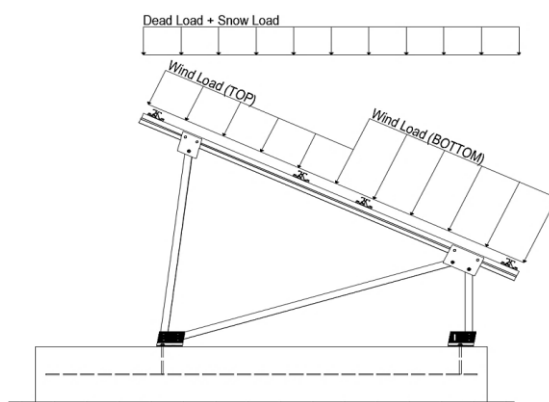
$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.39	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	108 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.742 k-ft
$M_z$ =	0.292 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>88%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.349 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.384 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>99%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

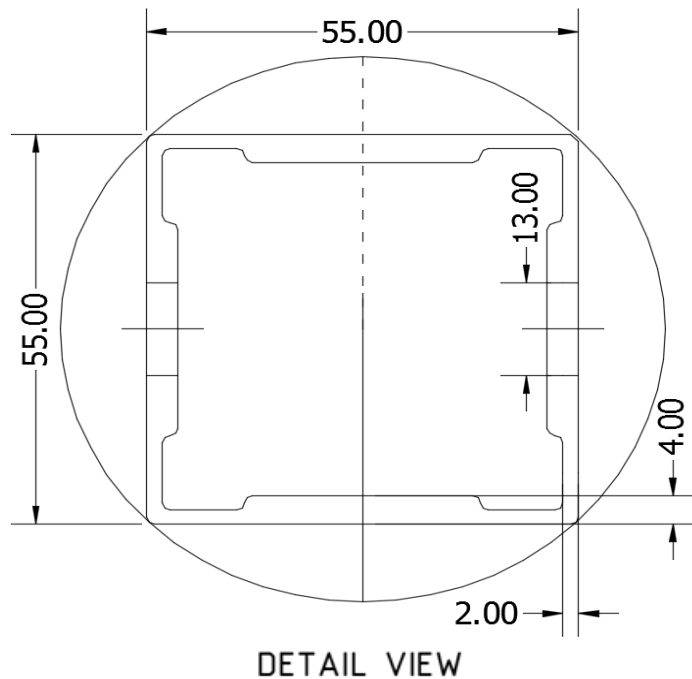
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.536 k-ft
$P_n$ =	0.662 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>40%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.012 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.485 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>26%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	-0.317 k-ft
$P_n$ =	0.690 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<u>28%</u>



DETAIL VIEW

### 5. FOUNDATION DESIGN CALCULATIONS

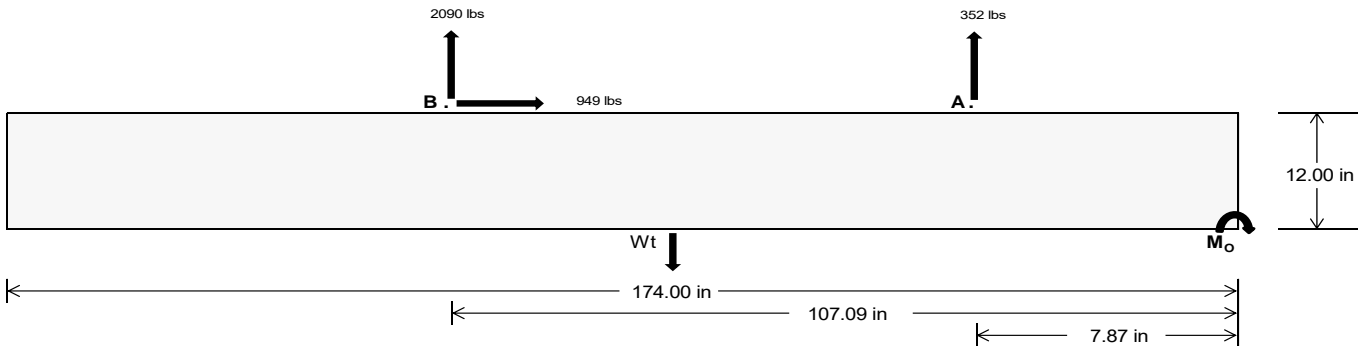
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>784.03</u>	<u>4550.94</u> k
Compressive Load =		<u>4252.95</u>	<u>4782.65</u> k
Lateral Load =		<u>353.67</u>	<u>2057.86</u> k
Moment (Weak Axis) =		<u>0.72</u>	<u>0.41</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 237987.5$  in-lbs  
Resisting Force Required = 2735.49 lbs  
S.F. = 1.67  
Weight Required = 4559.15 lbs  
Minimum Width = 27 in  
Weight Provided = 4730.63 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 174in long x 27in wide x 12in tall ballast foundation is required to resist overturning.

### Sliding

Force = 949.12 lbs  
Friction = 0.4  
Weight Required = 2372.80 lbs  
Resisting Weight = 4730.63 lbs  
Additional Weight Required = 0 lbs

Use a 174in long x 27in wide x 12in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 949.12 lbs  
Cohesion = 130 psf  
Area = 32.63 ft<sup>2</sup>  
Resisting = 2365.31 lbs  
Additional Weight Required = 0 lbs

Use a 174in long x 27in wide x 12in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure (Meyerhof, 1953)

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(14.5 \text{ ft})(1 \text{ ft})(2.25 \text{ ft}) =$   
27 in 28 in 29 in 30 in  
4731 lbs 4906 lbs 5081 lbs 5256 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in	27 in	28 in	29 in	30 in
$F_A$	1582 lbs	1582 lbs	1582 lbs	1582 lbs	1273 lbs	1273 lbs	1273 lbs	1273 lbs	2012 lbs	2012 lbs	2012 lbs	2012 lbs	-352 lbs	-352 lbs	-352 lbs	-352 lbs
$F_B$	1703 lbs	1703 lbs	1703 lbs	1703 lbs	1550 lbs	1550 lbs	1550 lbs	1550 lbs	2302 lbs	2302 lbs	2302 lbs	2302 lbs	-2090 lbs	-2090 lbs	-2090 lbs	-2090 lbs
$F_V$	170 lbs	170 lbs	170 lbs	170 lbs	855 lbs	855 lbs	855 lbs	855 lbs	756 lbs	756 lbs	756 lbs	756 lbs	-949 lbs	-949 lbs	-949 lbs	-949 lbs
$P_{total}$	8016 lbs	8192 lbs	8367 lbs	8542 lbs	7554 lbs	7729 lbs	7904 lbs	8080 lbs	9045 lbs	9220 lbs	9395 lbs	9571 lbs	396 lbs	501 lbs	607 lbs	712 lbs
$M$	7412 lbs-ft	7412 lbs-ft	7412 lbs-ft	7412 lbs-ft	4946 lbs-ft	4946 lbs-ft	4946 lbs-ft	4946 lbs-ft	8660 lbs-ft	8660 lbs-ft	8660 lbs-ft	8660 lbs-ft	2127 lbs-ft	2127 lbs-ft	2127 lbs-ft	2127 lbs-ft
$e$	0.92 ft	0.90 ft	0.89 ft	0.87 ft	0.65 ft	0.64 ft	0.63 ft	0.61 ft	0.96 ft	0.94 ft	0.92 ft	0.90 ft	5.37 ft	4.24 ft	3.51 ft	2.99 ft
$L'$	12.65 ft	12.69 ft	12.73 ft	12.76 ft	13.19 ft	13.22 ft	13.25 ft	13.28 ft	12.59 ft	12.62 ft	12.66 ft	12.69 ft	3.76 ft	6.02 ft	7.49 ft	8.52 ft
$A'$	28.5 sqft	29.6 sqft	30.8 sqft	31.9 sqft	29.7 sqft	30.8 sqft	32.0 sqft	33.2 sqft	28.3 sqft	29.5 sqft	30.6 sqft	31.7 sqft	8.5 sqft	14.0 sqft	18.1 sqft	21.3 sqft
$f_{meyerhof}$	281.6 psf	276.6 psf	272.0 psf	267.7 psf	254.5 psf	250.6 psf	246.9 psf	243.4 psf	319.4 psf	313.1 psf	307.2 psf	301.7 psf	46.8 psf	35.7 psf	33.5 psf	33.4 psf

Maximum Bearing Pressure = 319 psf  
Allowable Bearing Pressure = 1500 psf

Use a 174in long x 27in wide x 12in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

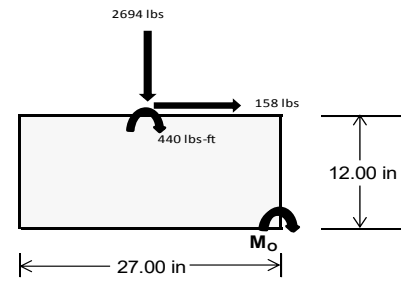
### Overturning Check

$M_o = 2432.4 \text{ ft-lbs}$   
 Resisting Force Required = 2162.13 lbs  
 S.F. = 1.67  
 Weight Required = 3603.55 lbs  
 Minimum Width = 27 in  
 Weight Provided = 4730.63 lbs

*A minimum 174in long x 27in wide x 12in tall ballast foundation is required to resist overturning.*

### Bearing Pressure (Meyerhof, 1953)

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	27 in			27 in			27 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	290 lbs	663 lbs	224 lbs	956 lbs	2694 lbs	904 lbs	108 lbs	194 lbs	42 lbs
$F_v$	222 lbs	217 lbs	226 lbs	164 lbs	158 lbs	177 lbs	223 lbs	219 lbs	224 lbs
$P_{total}$	6147 lbs	6519 lbs	6080 lbs	6531 lbs	8269 lbs	6479 lbs	1821 lbs	1906 lbs	1755 lbs
$M$	793 lbs-ft	782 lbs-ft	801 lbs-ft	598 lbs-ft	598 lbs-ft	633 lbs-ft	790 lbs-ft	779 lbs-ft	793 lbs-ft
$e$	0.13 ft	0.12 ft	0.13 ft	0.09 ft	0.07 ft	0.10 ft	0.43 ft	0.41 ft	0.45 ft
$B'$	1.99 ft	2.01 ft	1.99 ft	2.07 ft	2.11 ft	2.05 ft	1.38 ft	1.43 ft	1.35 ft
$A'$	28.9 sqft	29.1 sqft	28.8 sqft	30.0 sqft	30.5 sqft	29.8 sqft	20.0 sqft	20.8 sqft	19.5 sqft
$f_{meyerhof}$	212.8 psf	223.7 psf	211.1 psf	217.9 psf	270.9 psf	217.5 psf	90.8 psf	91.7 psf	89.9 psf



Maximum Bearing Pressure = 271 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 174in long x 27in wide x 12in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 174in long x 27in wide x 12in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.545 k
Allowable Uplift =	1.214 k
Utilization =	<u>45%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.730 k
Allowable Uplift =	4.357 k
Utilization =	<u>40%</u>



### 6.2 Strut Connections

The aluminum struts connect the front end of girder to a center section of the steel post. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.272 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

#### Rear Strut

Maximum Axial Load =	3.448 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>46%</u>

#### Diagonal Strut

Maximum Axial Load =	1.579 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>21%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>0.699 ≤ 1.038, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 108 \text{ in}$$

$$J = 0.432$$

$$298.779$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 108$$

$$J = 0.432$$

$$190.005$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.9$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

### Strong Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.7

$$\begin{aligned} \lambda &= 1.41345 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77788 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 13.6277 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-71.531	-71.531	0	0
2	M14	y	-71.531	-71.531	0	0
3	M15	y	-112.406	-112.406	0	0
4	M16	y	-112.406	-112.406	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	163.5	163.5	0	0
2	M14	y	125.35	125.35	0	0
3	M15	y	68.125	68.125	0	0
4	M16	y	68.125	68.125	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 72 Cell 2V 20° 115mph 30psf 9ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	76.717	1	885.685	3	186.313	1	.014	1	.294	1	1.54	1
20		min	3.882	12	-819.755	1	-110.782	14	-.002	3	.007	12	-1.588	3
21	11	max	76.717	1	675.624	1	-4.58	12	.014	1	.127	1	.793	1
22		min	3.882	12	-728.069	3	-146.871	1	0	15	.002	12	-.781	3
23	12	max	76.717	1	531.492	1	-3.152	12	.014	1	.054	4	.189	1
24		min	3.882	12	-570.454	3	-107.429	1	0	15	-.003	3	-.132	3
25	13	max	76.717	1	387.361	1	-1.724	12	.014	1	.025	5	.36	3
26		min	3.882	12	-412.838	3	-67.987	1	0	15	-.088	1	-.27	1
27	14	max	76.717	1	243.23	1	-.296	12	.014	1	0	15	.694	3
28		min	2.193	15	-255.223	3	-31.128	4	0	15	-.136	1	-.585	1
29	15	max	76.717	1	99.098	1	10.897	1	.014	1	-.005	12	.87	3
30		min	-8.448	5	-97.608	3	-22.553	5	0	15	-.145	1	-.757	1
31	16	max	76.717	1	60.008	3	50.339	1	.014	1	-.003	12	.889	3
32		min	-20.249	5	-45.033	1	-20.38	5	0	15	-.114	1	-.784	1
33	17	max	76.717	1	217.623	3	89.781	1	.014	1	0	3	.75	3
34		min	-32.049	5	-189.165	1	-18.207	5	0	15	-.076	4	-.667	1
35	18	max	76.717	1	375.238	3	129.223	1	.014	1	.065	1	.454	3
36		min	-43.85	5	-333.296	1	-16.033	5	0	15	-.082	5	-.405	1
37	19	max	76.717	1	532.854	3	168.665	1	.014	1	.214	1	0	1
38		min	-55.65	5	-477.427	1	-13.86	5	0	15	-.097	5	0	3
39	M14	1	max	56.561	4	529.876	1	-7.085	12	.009	.254	1	0	1
40		min	2.036	12	-426.053	3	-175.215	1	-.015	1	.012	12	0	3
41	2	max	44.76	4	385.745	1	-5.657	12	.009	3	.159	4	.366	3
42		min	2.036	12	-306.587	3	-135.773	1	-.015	1	.006	12	-.458	1
43	3	max	44.496	1	241.613	1	-4.229	12	.009	3	.092	5	.613	3
44		min	2.036	12	-187.122	3	-96.331	1	-.015	1	-.018	1	-.771	1
45	4	max	44.496	1	97.482	1	-2.801	12	.009	3	.051	5	.741	3
46		min	2.036	12	-67.657	3	-56.889	1	-.015	1	-.095	1	-.941	1
47	5	max	44.496	1	51.809	3	-1.233	10	.009	3	.013	5	.748	3
48		min	.067	15	-46.649	1	-42.166	4	-.015	1	-.132	1	-.966	1
49	6	max	44.496	1	171.274	3	21.995	1	.009	3	-.005	12	.637	3
50		min	-11.692	5	-190.781	1	-35.329	5	-.015	1	-.13	1	-.848	1
51	7	max	44.496	1	290.739	3	61.437	1	.009	3	-.004	12	.406	3
52		min	-23.493	5	-334.912	1	-33.155	5	-.015	1	-.088	1	-.585	1
53	8	max	44.496	1	410.205	3	100.879	1	.009	3	0	10	.055	3
54		min	-35.294	5	-479.044	1	-30.982	5	-.015	1	-.094	4	-.178	1
55	9	max	44.496	1	529.67	3	140.321	1	.009	3	.114	1	.373	1
56		min	-47.094	5	-623.175	1	-28.809	5	-.015	1	-.12	5	-.414	3
57	10	max	72.643	4	649.135	3	179.763	1	.015	1	.274	1	1.068	1
58		min	2.036	12	-767.306	1	-114.472	14	-.009	3	.007	12	-1.004	3
59	11	max	60.842	4	623.175	1	-4.338	12	.015	1	.159	4	.373	1
60		min	2.036	12	-529.67	3	-140.321	1	-.009	3	.002	12	-.414	3
61	12	max	49.042	4	479.044	1	-2.91	12	.015	1	.09	4	.055	3
62		min	2.036	12	-410.205	3	-100.879	1	-.009	3	-.007	1	-.178	1
63	13	max	44.496	1	334.912	1	-1.482	12	.015	1	.048	5	.406	3
64		min	2.036	12	-290.739	3	-61.437	1	-.009	3	-.088	1	-.585	1
65	14	max	44.496	1	190.781	1	.038	3	.015	1	.01	5	.637	3
66		min	2.036	12	-171.274	3	-43.128	4	-.009	3	-.13	1	-.848	1
67	15	max	44.496	1	46.649	1	17.447	1	.015	1	-.004	12	.748	3
68		min	2.036	12	-51.809	3	-35.532	5	-.009	3	-.132	1	-.966	1
69	16	max	44.496	1	67.657	3	56.889	1	.015	1	-.002	12	.741	3
70		min	-7.46	5	-97.482	1	-33.359	5	-.009	3	-.095	1	-.941	1
71	17	max	44.496	1	187.122	3	96.331	1	.015	1	.002	3	.613	3
72		min	-19.261	5	-241.613	1	-31.185	5	-.009	3	-.1	4	-.771	1
73	18	max	44.496	1	306.587	3	135.773	1	.015	1	.098	1	.366	3
74		min	-31.061	5	-385.745	1	-29.012	5	-.009	3	-.124	5	-.458	1
75	19	max	44.496	1	426.053	3	175.215	1	.015	1	.254	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76		min	-42.862	5	-529.876	1	-26.839	5	-.009	3	-.152	5	0	3
77	M15	max	88.81	5	598.817	1	-7.022	12	.015	1	.303	4	0	2
78		min	-47.367	1	-233.412	3	-175.17	1	-.008	3	.012	12	0	3
79		max	77.01	5	434.248	1	-5.594	12	.015	1	.211	4	.202	3
80		min	-47.367	1	-171.172	3	-135.728	1	-.008	3	.006	12	-.517	1
81		max	65.209	5	269.68	1	-4.166	12	.015	1	.129	5	.342	3
82		min	-47.367	1	-108.931	3	-96.286	1	-.008	3	-.018	1	-.868	1
83		max	53.409	5	105.111	1	-2.738	12	.015	1	.074	5	.42	3
84		min	-47.367	1	-46.691	3	-66.903	4	-.008	3	-.095	1	-1.056	1
85		max	41.608	5	15.549	3	-1.259	10	.015	1	.021	5	.436	3
86		min	-47.367	1	-59.458	1	-56.578	4	-.008	3	-.132	1	-1.079	1
87		max	29.808	5	77.79	3	22.04	1	.015	1	-.005	12	.389	3
88		min	-47.367	1	-224.027	1	-49.724	5	-.008	3	-.13	1	-.937	1
89		max	18.007	5	140.03	3	61.482	1	.015	1	-.004	12	.28	3
90		min	-47.367	1	-388.596	1	-47.551	5	-.008	3	-.098	4	-.631	1
91		max	6.207	5	202.27	3	100.924	1	.015	1	0	10	.109	3
92		min	-47.367	1	-553.165	1	-45.378	5	-.008	3	-.129	4	-.16	1
93		max	-2.441	12	264.511	3	140.366	1	.015	1	.114	1	.476	1
94		min	-47.367	1	-717.734	1	-43.204	5	-.008	3	-.169	5	-.124	3
95		max	-2.441	12	326.751	3	179.808	1	.008	3	.302	4	1.276	1
96		min	-47.367	1	-882.303	1	-121.809	14	-.015	1	.007	12	-.42	3
97		max	.084	15	717.734	1	-4.401	12	.008	3	.208	4	.476	1
98		min	-47.367	1	-264.511	3	-140.366	1	-.015	1	.002	12	-.124	3
99		max	-2.441	12	553.165	1	-2.973	12	.008	3	.125	4	.109	3
100		min	-47.367	1	-202.27	3	-100.924	1	-.015	1	-.007	1	-.16	1
101		max	-2.441	12	388.596	1	-1.545	12	.008	3	.069	5	.28	3
102		min	-47.367	1	-140.03	3	-67.896	4	-.015	1	-.088	1	-.631	1
103		max	-2.441	12	224.027	1	-.066	3	.008	3	.015	5	.389	3
104		min	-47.367	1	-77.79	3	-57.571	4	-.015	1	-.13	1	-.937	1
105		max	-2.441	12	59.458	1	17.402	1	.008	3	-.004	12	.436	3
106		min	-57.954	4	-15.549	3	-49.929	5	-.015	1	-.132	1	-1.079	1
107		max	-2.441	12	46.691	3	56.844	1	.008	3	-.002	12	.42	3
108		min	-69.755	4	-105.111	1	-47.756	5	-.015	1	-.106	4	-1.056	1
109		max	-2.441	12	108.931	3	96.286	1	.008	3	.002	3	.342	3
110		min	-81.555	4	-269.68	1	-45.582	5	-.015	1	-.137	4	-.868	1
111		max	-2.441	12	171.172	3	135.728	1	.008	3	.098	1	.202	3
112		min	-93.356	4	-434.248	1	-43.409	5	-.015	1	-.176	5	-.517	1
113		max	-2.441	12	233.412	3	175.17	1	.008	3	.253	1	0	2
114		min	-105.156	4	-598.817	1	-41.236	5	-.015	1	-.218	5	0	5
115	M16	max	84.586	5	547.015	1	-6.651	12	.012	1	.217	1	0	1
116		min	-85.179	1	-203.944	3	-169.118	1	-.01	3	.01	12	0	3
117		max	72.785	5	382.446	1	-5.223	12	.012	1	.143	4	.173	3
118		min	-85.179	1	-141.704	3	-129.676	1	-.01	3	.004	12	-.465	1
119		max	60.985	5	217.877	1	-3.795	12	.012	1	.087	5	.283	3
120		min	-85.179	1	-79.464	3	-90.234	1	-.01	3	-.043	1	-.765	1
121		max	49.184	5	53.308	1	-2.367	12	.012	1	.05	5	.332	3
122		min	-85.179	1	-17.223	3	-50.792	1	-.01	3	-.113	1	-.9	1
123		max	37.384	5	45.017	3	-.751	10	.012	1	.015	5	.318	3
124		min	-85.179	1	-111.261	1	-37.059	4	-.01	3	-.144	1	-.872	1
125		max	25.583	5	107.258	3	28.092	1	.012	1	-.005	12	.242	3
126		min	-85.179	1	-275.829	1	-31.812	5	-.01	3	-.136	1	-.678	1
127		max	13.783	5	169.498	3	67.534	1	.012	1	-.004	12	.103	3
128		min	-85.179	1	-440.398	1	-29.638	5	-.01	3	-.088	1	-.32	1
129		max	1.982	5	231.738	3	106.975	1	.012	1	.001	2	.203	1
130		min	-85.179	1	-604.967	1	-27.465	5	-.01	3	-.079	4	-.097	3
131		max	-4.007	12	293.979	3	146.417	1	.012	1	.126	1	.89	1
132		min	-85.179	1	-769.536	1	-25.292	5	-.01	3	-.103	5	-.36	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-4.007	12	356.219	3	185.859	1	.01	3	.292	1	1.742	1
134			min	-85.179	1	-934.105	1	-115.801	14	-.012	1	.008	12	-.685	3
135		11	max	.552	5	769.536	1	-4.772	12	.01	3	.145	4	.89	1
136			min	-85.179	1	-293.979	3	-146.417	1	-.012	1	.003	12	-.36	3
137		12	max	-4.007	12	604.967	1	-3.344	12	.01	3	.078	4	.203	1
138			min	-85.179	1	-231.738	3	-106.975	1	-.012	1	-.002	3	-.097	3
139		13	max	-4.007	12	440.398	1	-1.916	12	.01	3	.039	5	.103	3
140			min	-85.179	1	-169.498	3	-67.534	1	-.012	1	-.088	1	-.32	1
141		14	max	-4.007	12	275.829	1	-.488	12	.01	3	.003	5	.242	3
142			min	-85.179	1	-107.258	3	-41.281	4	-.012	1	-.136	1	-.678	1
143		15	max	-4.007	12	111.261	1	11.35	1	.01	3	-.005	12	.318	3
144			min	-85.179	1	-45.017	3	-32.714	5	-.012	1	-.144	1	-.872	1
145		16	max	-4.007	12	17.223	3	50.792	1	.01	3	-.003	12	.332	3
146			min	-85.179	1	-53.308	1	-30.54	5	-.012	1	-.113	1	-.9	1
147		17	max	-4.007	12	79.464	3	90.234	1	.01	3	0	3	.283	3
148			min	-88.966	4	-217.877	1	-28.367	5	-.012	1	-.102	4	-.765	1
149		18	max	-4.007	12	141.704	3	129.676	1	.01	3	.067	1	.173	3
150			min	-100.766	4	-382.446	1	-26.194	5	-.012	1	-.119	5	-.465	1
151		19	max	-4.007	12	203.944	3	169.118	1	.01	3	.217	1	0	1
152			min	-112.567	4	-547.015	1	-24.02	5	-.012	1	-.145	5	0	5
153	M2	1	max	1099.328	1	2.211	4	.906	1	0	3	0	3	0	1
154			min	-975.26	3	.543	15	-58.845	4	0	1	0	1	0	1
155		2	max	1099.744	1	2.202	4	.906	1	0	3	0	1	0	15
156			min	-974.948	3	.541	15	-59.205	4	0	1	-.017	4	0	4
157		3	max	1100.16	1	2.194	4	.906	1	0	3	0	1	0	15
158			min	-974.636	3	.539	15	-59.566	4	0	1	-.033	4	-.001	4
159		4	max	1100.575	1	2.185	4	.906	1	0	3	0	1	0	15
160			min	-974.324	3	.537	15	-59.926	4	0	1	-.05	4	-.002	4
161		5	max	1100.991	1	2.176	4	.906	1	0	3	.001	1	0	15
162			min	-974.012	3	.535	15	-60.287	4	0	1	-.067	4	-.002	4
163		6	max	1101.407	1	2.168	4	.906	1	0	3	.001	1	0	15
164			min	-973.7	3	.533	15	-60.647	4	0	1	-.084	4	-.003	4
165		7	max	1101.823	1	2.159	4	.906	1	0	3	.002	1	0	15
166			min	-973.388	3	.531	15	-61.008	4	0	1	-.101	4	-.004	4
167		8	max	1102.239	1	2.15	4	.906	1	0	3	.002	1	-.001	15
168			min	-973.077	3	.529	15	-61.368	4	0	1	-.118	4	-.004	4
169		9	max	1102.655	1	2.141	4	.906	1	0	3	.002	1	-.001	15
170			min	-972.765	3	.527	15	-61.729	4	0	1	-.135	4	-.005	4
171		10	max	1103.071	1	2.133	4	.906	1	0	3	.002	1	-.001	15
172			min	-972.453	3	.525	15	-62.089	4	0	1	-.153	4	-.005	4
173		11	max	1103.487	1	2.124	4	.906	1	0	3	.003	1	-.001	15
174			min	-972.141	3	.523	15	-62.45	4	0	1	-.17	4	-.006	4
175		12	max	1103.902	1	2.115	4	.906	1	0	3	.003	1	-.002	15
176			min	-971.829	3	.521	15	-62.81	4	0	1	-.188	4	-.007	4
177		13	max	1104.318	1	2.107	4	.906	1	0	3	.003	1	-.002	15
178			min	-971.517	3	.518	15	-63.171	4	0	1	-.205	4	-.007	4
179		14	max	1104.734	1	2.098	4	.906	1	0	3	.003	1	-.002	15
180			min	-971.205	3	.516	15	-63.531	4	0	1	-.223	4	-.008	4
181		15	max	1105.15	1	2.089	4	.906	1	0	3	.004	1	-.002	15
182			min	-970.893	3	.514	15	-63.892	4	0	1	-.241	4	-.008	4
183		16	max	1105.566	1	2.08	4	.906	1	0	3	.004	1	-.002	15
184			min	-970.581	3	.512	15	-64.252	4	0	1	-.259	4	-.009	4
185		17	max	1105.982	1	2.072	4	.906	1	0	3	.004	1	-.002	15
186			min	-970.269	3	.51	15	-64.613	4	0	1	-.277	4	-.01	4
187		18	max	1106.398	1	2.063	4	.906	1	0	3	.004	1	-.003	15
188			min	-969.957	3	.508	15	-64.973	4	0	1	-.295	4	-.01	4
189		19	max	1106.814	1	2.054	4	.906	1	0	3	.005	1	-.003	15





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-969.646	3	.506	15	-65.334	4	0	1	-.313	4	-.011	4
191	M3	1	max	387.1	2	9.133	4	.21	1	0	12	0	.011	4
192		min	-515.886	3	2.161	15	-3.415	5	0	4	-.005	4	.003	15
193		2	max	386.93	2	8.259	4	.21	1	0	12	0	.007	4
194		min	-516.014	3	1.955	15	-2.806	5	0	4	-.007	4	.002	12
195		3	max	386.759	2	7.384	4	.21	1	0	12	0	.003	2
196		min	-516.142	3	1.75	15	-2.197	5	0	4	-.008	4	0	12
197		4	max	386.589	2	6.51	4	.21	1	0	12	0	0	2
198		min	-516.27	3	1.544	15	-1.589	5	0	4	-.009	4	-.002	3
199		5	max	386.419	2	5.635	4	.21	1	0	12	0	0	15
200		min	-516.397	3	1.338	15	-.98	5	0	4	-.009	5	-.003	6
201		6	max	386.248	2	4.761	4	.21	1	0	12	0	1	15
202		min	-516.525	3	1.133	15	-.371	5	0	4	-.01	5	-.006	6
203		7	max	386.078	2	3.886	4	.299	4	0	12	0	1	15
204		min	-516.653	3	.927	15	.01	12	0	4	-.01	5	-.008	6
205		8	max	385.907	2	3.012	4	.907	4	0	12	0	1	15
206		min	-516.781	3	.722	15	.01	12	0	4	-.009	5	-.009	6
207		9	max	385.737	2	2.137	4	1.516	4	0	12	0	1	15
208		min	-516.908	3	.516	15	.01	12	0	4	-.009	5	-.011	6
209		10	max	385.567	2	1.263	4	2.125	4	0	12	0	1	15
210		min	-517.036	3	.311	15	.01	12	0	4	-.008	5	-.011	6
211		11	max	385.396	2	.399	2	2.733	4	0	12	.001	1	15
212		min	-517.164	3	.037	12	.01	12	0	4	-.007	5	-.012	6
213		12	max	385.226	2	-.1	15	3.342	4	0	12	.001	1	15
214		min	-517.292	3	-.487	6	.01	12	0	4	-.006	5	-.012	6
215		13	max	385.056	2	-.306	15	3.951	4	0	12	.001	1	15
216		min	-517.419	3	-1.362	6	.01	12	0	4	-.004	5	-.011	6
217		14	max	384.885	2	-.512	15	4.56	4	0	12	.001	1	15
218		min	-517.547	3	-2.236	6	.01	12	0	4	-.002	5	-.01	6
219		15	max	384.715	2	-.717	15	5.168	4	0	12	.001	1	15
220		min	-517.675	3	-3.111	6	.01	12	0	4	0	12	-.009	6
221		16	max	384.545	2	-.923	15	5.777	4	0	12	.003	4	15
222		min	-517.803	3	-3.985	6	.01	12	0	4	0	12	-.008	6
223		17	max	384.374	2	-1.128	15	6.386	4	0	12	.006	4	15
224		min	-517.93	3	-4.859	6	.01	12	0	4	0	12	-.005	6
225		18	max	384.204	2	-1.334	15	6.994	4	0	12	.009	4	15
226		min	-518.058	3	-5.734	6	.01	12	0	4	0	12	-.003	6
227		19	max	384.034	2	-1.539	15	7.603	4	0	12	.013	4	1
228		min	-518.186	3	-6.608	6	.01	12	0	4	0	12	0	1
229	M4	1	max	1177.604	1	0	1	-.587	12	0	1	.008	4	1
230		min	-170.254	3	0	1	-.270.895	4	0	1	0	12	0	1
231		2	max	1177.775	1	0	1	-.587	12	0	1	0	12	1
232		min	-170.126	3	0	1	-.271.043	4	0	1	-.023	4	0	1
233		3	max	1177.945	1	0	1	-.587	12	0	1	0	12	1
234		min	-169.998	3	0	1	-.271.191	4	0	1	-.055	4	0	1
235		4	max	1178.115	1	0	1	-.587	12	0	1	0	12	1
236		min	-169.871	3	0	1	-.271.338	4	0	1	-.086	4	0	1
237		5	max	1178.286	1	0	1	-.587	12	0	1	0	12	1
238		min	-169.743	3	0	1	-.271.486	4	0	1	-.117	4	0	1
239		6	max	1178.456	1	0	1	-.587	12	0	1	0	12	1
240		min	-169.615	3	0	1	-.271.634	4	0	1	-.148	4	0	1
241		7	max	1178.626	1	0	1	-.587	12	0	1	0	12	1
242		min	-169.487	3	0	1	-.271.781	4	0	1	-.179	4	0	1
243		8	max	1178.797	1	0	1	-.587	12	0	1	0	12	1
244		min	-169.359	3	0	1	-.271.929	4	0	1	-.211	4	0	1
245		9	max	1178.967	1	0	1	-.587	12	0	1	0	12	1
246		min	-169.232	3	0	1	-.272.077	4	0	1	-.242	4	0	1





Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1179.137	1	0	1	-587	12	0	1	0	12	0	1
248			min	-169.104	3	0	1	-272.224	4	0	1	-.273	4	0	1
249		11	max	1179.308	1	0	1	-587	12	0	1	0	12	0	1
250			min	-168.976	3	0	1	-272.372	4	0	1	-.304	4	0	1
251		12	max	1179.478	1	0	1	-587	12	0	1	0	12	0	1
252			min	-168.848	3	0	1	-272.519	4	0	1	-.336	4	0	1
253		13	max	1179.648	1	0	1	-587	12	0	1	0	12	0	1
254			min	-168.721	3	0	1	-272.667	4	0	1	-.367	4	0	1
255		14	max	1179.819	1	0	1	-587	12	0	1	0	12	0	1
256			min	-168.593	3	0	1	-272.815	4	0	1	-.398	4	0	1
257		15	max	1179.989	1	0	1	-587	12	0	1	0	12	0	1
258			min	-168.465	3	0	1	-272.962	4	0	1	-.43	4	0	1
259		16	max	1180.159	1	0	1	-587	12	0	1	0	12	0	1
260			min	-168.337	3	0	1	-273.11	4	0	1	-.461	4	0	1
261		17	max	1180.33	1	0	1	-587	12	0	1	-.001	12	0	1
262			min	-168.21	3	0	1	-273.258	4	0	1	-.492	4	0	1
263		18	max	1180.5	1	0	1	-587	12	0	1	-.001	12	0	1
264			min	-168.082	3	0	1	-273.405	4	0	1	-.524	4	0	1
265		19	max	1180.671	1	0	1	-587	12	0	1	-.001	12	0	1
266			min	-167.954	3	0	1	-273.553	4	0	1	-.555	4	0	1
267	M6	1	max	3440.295	1	2.428	2	0	1	0	1	0	4	0	1
268			min	-3129.045	3	.31	12	-59.48	4	0	4	0	1	0	1
269		2	max	3440.711	1	2.422	2	0	1	0	1	0	1	0	12
270			min	-3128.733	3	.306	12	-59.841	4	0	4	-.017	4	0	2
271		3	max	3441.127	1	2.415	2	0	1	0	1	0	1	0	12
272			min	-3128.421	3	.303	12	-60.201	4	0	4	-.034	4	-.001	2
273		4	max	3441.543	1	2.408	2	0	1	0	1	0	1	0	12
274			min	-3128.109	3	.3	12	-60.562	4	0	4	-.05	4	-.002	2
275		5	max	3441.959	1	2.401	2	0	1	0	1	0	1	0	12
276			min	-3127.797	3	.296	12	-60.922	4	0	4	-.068	4	-.003	2
277		6	max	3442.375	1	2.394	2	0	1	0	1	0	1	0	12
278			min	-3127.485	3	.293	12	-61.283	4	0	4	-.085	4	-.003	2
279		7	max	3442.791	1	2.388	2	0	1	0	1	0	1	0	12
280			min	-3127.173	3	.289	12	-61.643	4	0	4	-.102	4	-.004	2
281		8	max	3443.206	1	2.381	2	0	1	0	1	0	1	0	12
282			min	-3126.861	3	.286	12	-62.004	4	0	4	-.119	4	-.005	2
283		9	max	3443.622	1	2.374	2	0	1	0	1	0	1	0	12
284			min	-3126.549	3	.283	12	-62.364	4	0	4	-.137	4	-.005	2
285		10	max	3444.038	1	2.367	2	0	1	0	1	0	1	0	12
286			min	-3126.237	3	.279	12	-62.725	4	0	4	-.154	4	-.006	2
287		11	max	3444.454	1	2.36	2	0	1	0	1	0	1	0	12
288			min	-3125.926	3	.276	12	-63.085	4	0	4	-.172	4	-.007	2
289		12	max	3444.87	1	2.354	2	0	1	0	1	0	1	0	12
290			min	-3125.614	3	.272	12	-63.446	4	0	4	-.19	4	-.007	2
291		13	max	3445.286	1	2.347	2	0	1	0	1	0	1	0	12
292			min	-3125.302	3	.269	12	-63.806	4	0	4	-.207	4	-.008	2
293		14	max	3445.702	1	2.34	2	0	1	0	1	0	1	-.001	12
294			min	-3124.99	3	.266	12	-64.167	4	0	4	-.225	4	-.009	2
295		15	max	3446.118	1	2.333	2	0	1	0	1	0	1	-.001	12
296			min	-3124.678	3	.262	12	-64.527	4	0	4	-.243	4	-.009	2
297		16	max	3446.533	1	2.327	2	0	1	0	1	0	1	-.001	12
298			min	-3124.366	3	.259	12	-64.888	4	0	4	-.262	4	-.01	2
299		17	max	3446.949	1	2.32	2	0	1	0	1	0	1	-.001	12
300			min	-3124.054	3	.255	12	-65.248	4	0	4	-.28	4	-.011	2
301		18	max	3447.365	1	2.313	2	0	1	0	1	0	1	-.001	12
302			min	-3123.742	3	.252	12	-65.609	4	0	4	-.298	4	-.011	2
303		19	max	3447.781	1	2.306	2	0	1	0	1	0	1	-.001	12



Company : Schletter, Inc.  
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Nov 3, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-3123.43	3	.249	12	-65.969	4	0	4	-.317	4	-.012	2
305	M7	1	max	1485.025	2	9.141	6	0	1	0	0	1	.012	2
306		min	-1576.74	3	2.145	15	-3.631	5	0	4	-.005	4	.001	12
307		2	max	1484.854	2	8.267	6	0	1	0	0	1	.009	2
308		min	-1576.867	3	1.939	15	-3.023	5	0	4	-.007	4	0	3
309		3	max	1484.684	2	7.392	6	0	1	0	0	1	.005	2
310		min	-1576.995	3	1.734	15	-2.414	5	0	4	-.008	4	-.002	3
311		4	max	1484.514	2	6.518	6	0	1	0	0	1	.003	2
312		min	-1577.123	3	1.528	15	-1.805	5	0	4	-.009	4	-.004	3
313		5	max	1484.343	2	5.643	6	0	1	0	0	1	0	2
314		min	-1577.251	3	1.323	15	-1.197	5	0	4	-.01	4	-.005	3
315		6	max	1484.173	2	4.769	6	0	1	0	0	1	-.001	15
316		min	-1577.378	3	1.117	15	-.588	5	0	4	-.01	4	-.007	3
317		7	max	1484.003	2	3.895	6	.031	4	0	0	1	-.002	15
318		min	-1577.506	3	.912	15	0	1	0	4	-.01	4	-.008	4
319		8	max	1483.832	2	3.02	6	.639	4	0	0	1	-.002	15
320		min	-1577.634	3	.706	15	0	1	0	4	-.01	4	-.009	4
321		9	max	1483.662	2	2.146	6	1.248	4	0	0	1	-.002	15
322		min	-1577.762	3	.5	15	0	1	0	4	-.01	4	-.011	4
323		10	max	1483.491	2	1.402	2	1.857	4	0	0	1	-.003	15
324		min	-1577.89	3	.165	12	0	1	0	4	-.009	4	-.011	4
325		11	max	1483.321	2	.721	2	2.466	4	0	0	1	-.003	15
326		min	-1578.017	3	-.316	3	0	1	0	4	-.008	4	-.012	4
327		12	max	1483.151	2	.039	2	3.074	4	0	0	1	-.003	15
328		min	-1578.145	3	-.827	3	0	1	0	4	-.007	4	-.012	4
329		13	max	1482.98	2	-.322	15	3.683	4	0	0	1	-.003	15
330		min	-1578.273	3	-1.352	4	0	1	0	4	-.005	4	-.011	4
331		14	max	1482.81	2	-.527	15	4.292	4	0	0	1	-.002	15
332		min	-1578.401	3	-2.227	4	0	1	0	4	-.003	4	-.01	4
333		15	max	1482.64	2	-.733	15	4.9	4	0	0	1	-.002	15
334		min	-1578.528	3	-3.101	4	0	1	0	4	-.001	4	-.009	4
335		16	max	1482.469	2	-.938	15	5.509	4	0	0	1	-.002	15
336		min	-1578.656	3	-3.975	4	0	1	0	4	0	1	-.008	4
337		17	max	1482.299	2	-1.144	15	6.118	4	0	0	1	-.001	15
338		min	-1578.784	3	-4.85	4	0	1	0	4	0	1	-.005	4
339		18	max	1482.129	2	-1.35	15	6.726	4	0	0	1	0	15
340		min	-1578.912	3	-5.724	4	0	1	0	4	0	1	-.003	4
341		19	max	1481.958	2	-1.555	15	7.335	4	0	0	1	.011	5
342		min	-1579.039	3	-6.599	4	0	1	0	4	0	1	0	1
343	M8	1	max	3268.434	1	0	1	0	1	0	0	5	0	1
344		min	-605.398	3	0	1	-260.932	4	0	1	0	1	0	1
345		2	max	3268.604	1	0	1	0	1	0	0	1	0	1
346		min	-605.27	3	0	1	-261.08	4	0	1	-.024	4	0	1
347		3	max	3268.774	1	0	1	0	1	0	0	1	0	1
348		min	-605.143	3	0	1	-261.228	4	0	1	-.054	4	0	1
349		4	max	3268.945	1	0	1	0	1	0	0	1	0	1
350		min	-605.015	3	0	1	-261.375	4	0	1	-.084	4	0	1
351		5	max	3269.115	1	0	1	0	1	0	0	1	0	1
352		min	-604.887	3	0	1	-261.523	4	0	1	-.114	4	0	1
353		6	max	3269.285	1	0	1	0	1	0	0	1	0	1
354		min	-604.759	3	0	1	-261.671	4	0	1	-.144	4	0	1
355		7	max	3269.456	1	0	1	0	1	0	0	1	0	1
356		min	-604.631	3	0	1	-261.818	4	0	1	-.174	4	0	1
357		8	max	3269.626	1	0	1	0	1	0	0	1	0	1
358		min	-604.504	3	0	1	-261.966	4	0	1	-.204	4	0	1
359		9	max	3269.796	1	0	1	0	1	0	0	1	0	1
360		min	-604.376	3	0	1	-262.114	4	0	1	-.234	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3269.967	1	0	1	0	1	0	1	0	1	0	1
362			min	-604.248	3	0	1	-262.261	4	0	1	-.264	4	0	1
363		11	max	3270.137	1	0	1	0	1	0	1	0	1	0	1
364			min	-604.12	3	0	1	-262.409	4	0	1	-.294	4	0	1
365		12	max	3270.307	1	0	1	0	1	0	1	0	1	0	1
366			min	-603.993	3	0	1	-262.556	4	0	1	-.324	4	0	1
367		13	max	3270.478	1	0	1	0	1	0	1	0	1	0	1
368			min	-603.865	3	0	1	-262.704	4	0	1	-.355	4	0	1
369		14	max	3270.648	1	0	1	0	1	0	1	0	1	0	1
370			min	-603.737	3	0	1	-262.852	4	0	1	-.385	4	0	1
371		15	max	3270.818	1	0	1	0	1	0	1	0	1	0	1
372			min	-603.609	3	0	1	-262.999	4	0	1	-.415	4	0	1
373		16	max	3270.989	1	0	1	0	1	0	1	0	1	0	1
374			min	-603.482	3	0	1	-263.147	4	0	1	-.445	4	0	1
375		17	max	3271.159	1	0	1	0	1	0	1	0	1	0	1
376			min	-603.354	3	0	1	-263.295	4	0	1	-.475	4	0	1
377		18	max	3271.33	1	0	1	0	1	0	1	0	1	0	1
378			min	-603.226	3	0	1	-263.442	4	0	1	-.506	4	0	1
379		19	max	3271.5	1	0	1	0	1	0	1	0	1	0	1
380			min	-603.098	3	0	1	-263.59	4	0	1	-.536	4	0	1
381	M10	1	max	1099.328	1	2.103	6	-.044	12	0	1	0	4	0	1
382			min	-975.26	3	.471	15	-59.323	4	0	5	0	3	0	1
383		2	max	1099.744	1	2.095	6	-.044	12	0	1	0	10	0	15
384			min	-974.948	3	.469	15	-59.683	4	0	5	-.017	4	0	6
385		3	max	1100.16	1	2.086	6	-.044	12	0	1	0	12	0	15
386			min	-974.636	3	.467	15	-60.044	4	0	5	-.033	4	-.001	6
387		4	max	1100.575	1	2.077	6	-.044	12	0	1	0	12	0	15
388			min	-974.324	3	.465	15	-60.404	4	0	5	-.05	4	-.002	6
389		5	max	1100.991	1	2.068	6	-.044	12	0	1	0	12	0	15
390			min	-974.012	3	.463	15	-60.764	4	0	5	-.067	4	-.002	6
391		6	max	1101.407	1	2.06	6	-.044	12	0	1	0	12	0	15
392			min	-973.7	3	.461	15	-61.125	4	0	5	-.084	4	-.003	6
393		7	max	1101.823	1	2.051	6	-.044	12	0	1	0	12	0	15
394			min	-973.388	3	.459	15	-61.485	4	0	5	-.102	4	-.003	6
395		8	max	1102.239	1	2.042	6	-.044	12	0	1	0	12	0	15
396			min	-973.077	3	.457	15	-61.846	4	0	5	-.119	4	-.004	6
397		9	max	1102.655	1	2.034	6	-.044	12	0	1	0	12	-.001	15
398			min	-972.765	3	.455	15	-62.206	4	0	5	-.136	4	-.005	6
399		10	max	1103.071	1	2.025	6	-.044	12	0	1	0	12	-.001	15
400			min	-972.453	3	.452	15	-62.567	4	0	5	-.154	4	-.005	6
401		11	max	1103.487	1	2.016	6	-.044	12	0	1	0	12	-.001	15
402			min	-972.141	3	.45	15	-62.927	4	0	5	-.171	4	-.006	6
403		12	max	1103.902	1	2.007	6	-.044	12	0	1	0	12	-.001	15
404			min	-971.829	3	.448	15	-63.288	4	0	5	-.189	4	-.006	6
405		13	max	1104.318	1	1.999	6	-.044	12	0	1	0	12	-.002	15
406			min	-971.517	3	.446	15	-63.648	4	0	5	-.207	4	-.007	6
407		14	max	1104.734	1	1.99	6	-.044	12	0	1	0	12	-.002	15
408			min	-971.205	3	.444	15	-64.009	4	0	5	-.225	4	-.007	6
409		15	max	1105.15	1	1.981	6	-.044	12	0	1	0	12	-.002	15
410			min	-970.893	3	.442	15	-64.369	4	0	5	-.243	4	-.008	6
411		16	max	1105.566	1	1.973	6	-.044	12	0	1	0	12	-.002	15
412			min	-970.581	3	.44	15	-64.73	4	0	5	-.261	4	-.009	6
413		17	max	1105.982	1	1.964	6	-.044	12	0	1	0	12	-.002	15
414			min	-970.269	3	.438	15	-65.09	4	0	5	-.279	4	-.009	6
415		18	max	1106.398	1	1.955	6	-.044	12	0	1	0	12	-.002	15
416			min	-969.957	3	.436	15	-65.451	4	0	5	-.297	4	-.01	6
417		19	max	1106.814	1	1.946	6	-.044	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418		min	-969.646	3	.434	15	-65.811	4	0	5	-.316	4	-.01	6
419	M11	max	387.1	2	9.069	6	-.01	12	0	1	0	12	.01	6
420		min	-515.886	3	2.118	15	-3.464	4	0	4	-.005	4	.002	15
421		max	386.93	2	8.195	6	-.01	12	0	1	0	12	.006	6
422		min	-516.014	3	1.912	15	-2.855	4	0	4	-.007	4	.001	15
423		max	386.759	2	7.32	6	-.01	12	0	1	0	12	.003	2
424		min	-516.142	3	1.707	15	-2.246	4	0	4	-.008	4	0	12
425		max	386.589	2	6.446	6	-.01	12	0	1	0	12	0	2
426		min	-516.27	3	1.501	15	-1.638	4	0	4	-.009	4	-.002	3
427		max	386.419	2	5.571	6	-.01	12	0	1	0	12	0	15
428		min	-516.397	3	1.296	15	-1.029	4	0	4	-.01	4	-.004	4
429		max	386.248	2	4.697	6	-.01	12	0	1	0	12	-.002	15
430		min	-516.525	3	1.09	15	-.42	4	0	4	-.01	4	-.006	4
431		max	386.078	2	3.822	6	.215	5	0	1	0	12	-.002	15
432		min	-516.653	3	.885	15	-.21	1	0	4	-.01	4	-.008	4
433		max	385.907	2	2.948	6	.824	5	0	1	0	12	-.002	15
434		min	-516.781	3	.679	15	-.21	1	0	4	-.01	4	-.01	4
435		max	385.737	2	2.074	6	1.432	5	0	1	0	12	-.003	15
436		min	-516.908	3	.473	15	-.21	1	0	4	-.009	4	-.011	4
437		max	385.567	2	1.199	6	2.041	5	0	1	0	12	-.003	15
438		min	-517.036	3	.268	15	-.21	1	0	4	-.008	4	-.012	4
439		max	385.396	2	.399	2	2.65	5	0	1	0	12	-.003	15
440		min	-517.164	3	.037	12	-.21	1	0	4	-.007	4	-.012	4
441		max	385.226	2	-.143	15	3.259	5	0	1	0	12	-.003	15
442		min	-517.292	3	-.551	4	-.21	1	0	4	-.006	4	-.012	4
443		max	385.056	2	-.349	15	3.867	5	0	1	0	12	-.003	15
444		min	-517.419	3	-1.425	4	-.21	1	0	4	-.004	4	-.012	4
445		max	384.885	2	-.554	15	4.476	5	0	1	0	12	-.003	15
446		min	-517.547	3	-2.3	4	-.21	1	0	4	-.002	4	-.011	4
447		max	384.715	2	-.76	15	5.085	5	0	1	0	5	-.002	15
448		min	-517.675	3	-3.174	4	-.21	1	0	4	-.001	1	-.009	4
449		max	384.545	2	-.965	15	5.693	5	0	1	.003	5	-.002	15
450		min	-517.803	3	-4.049	4	-.21	1	0	4	-.002	1	-.008	4
451		max	384.374	2	-1.171	15	6.302	5	0	1	.006	5	-.001	15
452		min	-517.93	3	-4.923	4	-.21	1	0	4	-.002	1	-.005	4
453		max	384.204	2	-1.376	15	6.911	5	0	1	.009	5	0	15
454		min	-518.058	3	-5.798	4	-.21	1	0	4	-.002	1	-.003	4
455		max	384.034	2	-1.582	15	7.52	5	0	1	.012	5	0	1
456		min	-518.186	3	-6.672	4	-.21	1	0	4	-.002	1	0	1
457	M12	max	1177.604	1	0	1	12.717	1	0	1	.007	5	0	1
458		min	-170.254	3	0	1	-264.151	4	0	1	-.001	1	0	1
459		max	1177.775	1	0	1	12.717	1	0	1	0	1	0	1
460		min	-170.126	3	0	1	-264.299	4	0	1	-.023	4	0	1
461		max	1177.945	1	0	1	12.717	1	0	1	.002	1	0	1
462		min	-169.998	3	0	1	-264.447	4	0	1	-.054	4	0	1
463		max	1178.115	1	0	1	12.717	1	0	1	.003	1	0	1
464		min	-169.871	3	0	1	-264.594	4	0	1	-.084	4	0	1
465		max	1178.286	1	0	1	12.717	1	0	1	.005	1	0	1
466		min	-169.743	3	0	1	-264.742	4	0	1	-.114	4	0	1
467		max	1178.456	1	0	1	12.717	1	0	1	.006	1	0	1
468		min	-169.615	3	0	1	-264.89	4	0	1	-.145	4	0	1
469		max	1178.626	1	0	1	12.717	1	0	1	.008	1	0	1
470		min	-169.487	3	0	1	-265.037	4	0	1	-.175	4	0	1
471		max	1178.797	1	0	1	12.717	1	0	1	.009	1	0	1
472		min	-169.359	3	0	1	-265.185	4	0	1	-.206	4	0	1
473		max	1178.967	1	0	1	12.717	1	0	1	.011	1	0	1
474		min	-169.232	3	0	1	-265.332	4	0	1	-.236	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1179.137	1	0	1	12.717	1	0	1	.012	1	0	1
476		min	-169.104	3	0	1	-265.48	4	0	1	-.267	4	0	1
477	11	max	1179.308	1	0	1	12.717	1	0	1	.014	1	0	1
478		min	-168.976	3	0	1	-265.628	4	0	1	-.297	4	0	1
479	12	max	1179.478	1	0	1	12.717	1	0	1	.015	1	0	1
480		min	-168.848	3	0	1	-265.775	4	0	1	-.328	4	0	1
481	13	max	1179.648	1	0	1	12.717	1	0	1	.016	1	0	1
482		min	-168.721	3	0	1	-265.923	4	0	1	-.358	4	0	1
483	14	max	1179.819	1	0	1	12.717	1	0	1	.018	1	0	1
484		min	-168.593	3	0	1	-266.071	4	0	1	-.389	4	0	1
485	15	max	1179.989	1	0	1	12.717	1	0	1	.019	1	0	1
486		min	-168.465	3	0	1	-266.218	4	0	1	-.419	4	0	1
487	16	max	1180.159	1	0	1	12.717	1	0	1	.021	1	0	1
488		min	-168.337	3	0	1	-266.366	4	0	1	-.45	4	0	1
489	17	max	1180.33	1	0	1	12.717	1	0	1	.022	1	0	1
490		min	-168.21	3	0	1	-266.514	4	0	1	-.48	4	0	1
491	18	max	1180.5	1	0	1	12.717	1	0	1	.024	1	0	1
492		min	-168.082	3	0	1	-266.661	4	0	1	-.511	4	0	1
493	19	max	1180.671	1	0	1	12.717	1	0	1	.025	1	0	1
494		min	-167.954	3	0	1	-266.809	4	0	1	-.542	4	0	1
495	M1	1	max	168.67	1	532.823	3	55.613	5	0	.214	1	0	15
496		min	-13.86	5	-475.27	1	-76.595	1	0	3	-.097	5	-.014	1
497	2	max	169.246	1	531.636	3	57.073	5	0	1	.167	1	.281	1
498		min	-13.591	5	-476.853	1	-76.595	1	0	3	-.062	5	-.332	3
499	3	max	330.584	3	556.181	1	1.297	5	0	3	.119	1	.566	1
500		min	-224.245	2	-393.223	3	-75.945	1	0	1	-.027	5	-.652	3
501	4	max	331.016	3	554.598	1	2.757	5	0	3	.072	1	.221	1
502		min	-223.669	2	-394.41	3	-75.945	1	0	1	-.026	5	-.407	3
503	5	max	331.448	3	553.015	1	4.217	5	0	3	.025	1	-.005	15
504		min	-223.093	2	-395.597	3	-75.945	1	0	1	-.024	5	-.162	3
505	6	max	331.88	3	551.432	1	5.678	5	0	3	-.001	12	.084	3
506		min	-222.517	2	-396.785	3	-75.945	1	0	1	-.025	4	-.465	1
507	7	max	332.313	3	549.848	1	7.138	5	0	3	-.003	12	.331	3
508		min	-221.941	2	-397.972	3	-75.945	1	0	1	-.069	1	-.807	1
509	8	max	332.745	3	548.265	1	8.598	5	0	3	-.006	12	.578	3
510		min	-221.364	2	-399.16	3	-75.945	1	0	1	-.116	1	-1.147	1
511	9	max	343.924	3	35.288	2	52.783	5	0	9	.073	1	.676	3
512		min	-152.515	2	.475	15	-120.49	1	0	3	-.142	5	-1.306	1
513	10	max	344.356	3	33.705	2	54.243	5	0	9	0	10	.659	3
514		min	-151.939	2	-.006	5	-120.49	1	0	3	-.109	4	-1.318	1
515	11	max	344.788	3	32.121	2	55.703	5	0	9	-.004	12	.643	3
516		min	-151.363	2	-1.977	4	-120.49	1	0	3	-.09	4	-1.329	1
517	12	max	355.863	3	260.249	3	151.361	5	0	1	.114	1	.561	3
518		min	-94.788	5	-586.552	1	-73.163	1	0	3	-.234	5	-1.174	1
519	13	max	356.296	3	259.062	3	152.821	5	0	1	.069	1	.4	3
520		min	-94.519	5	-588.135	1	-73.163	1	0	3	-.14	5	-.809	1
521	14	max	356.728	3	257.874	3	154.281	5	0	1	.023	1	.24	3
522		min	-94.25	5	-589.718	1	-73.163	1	0	3	-.044	5	-.444	1
523	15	max	357.16	3	256.687	3	155.741	5	0	1	.052	5	.08	3
524		min	-93.982	5	-591.302	1	-73.163	1	0	3	-.022	1	-.077	1
525	16	max	357.592	3	255.5	3	157.202	5	0	1	.149	5	.29	2
526		min	-93.713	5	-592.885	1	-73.163	1	0	3	-.068	1	-.079	3
527	17	max	358.024	3	254.312	3	158.662	5	0	1	.247	5	.659	1
528		min	-93.444	5	-594.468	1	-73.163	1	0	3	-.113	1	-.237	3
529	18	max	23.751	5	550.643	1	-4.007	12	0	5	.203	5	.329	1
530		min	-169.69	1	-202.821	3	-114.076	4	0	1	-.164	1	-1.117	3
531	19	max	24.02	5	549.06	1	-4.007	12	0	5	.145	5	.01	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532			min	-169.114	1	-204.009	3	-112.616	4	0	1	-.217	1	-.012	1
533	M5	1	max	372.615	1	1771.315	3	92.765	5	0	1	0	1	.029	1
534			min	12.016	12	-1629.299	1	0	1	0	4	-.204	4	0	15
535		2	max	373.191	1	1770.127	3	94.226	5	0	1	0	1	1.041	1
536			min	12.304	12	-1630.882	1	0	1	0	4	-.146	4	-1.096	3
537		3	max	1039.71	3	1583.667	1	42.373	4	0	4	0	1	2.018	1
538			min	-762.014	2	-1206.521	3	0	1	0	1	-.089	4	-2.161	3
539		4	max	1040.142	3	1582.084	1	43.833	4	0	4	0	1	1.036	1
540			min	-761.438	2	-1207.708	3	0	1	0	1	-.062	4	-1.412	3
541		5	max	1040.574	3	1580.5	1	45.294	4	0	4	0	1	.054	1
542			min	-760.862	2	-1208.895	3	0	1	0	1	-.034	4	-.662	3
543		6	max	1041.006	3	1578.917	1	46.754	4	0	4	0	1	.089	3
544			min	-760.285	2	-1210.083	3	0	1	0	1	-.006	5	-.926	1
545		7	max	1041.438	3	1577.334	1	48.214	4	0	4	.024	4	.84	3
546			min	-759.709	2	-1211.27	3	0	1	0	1	0	1	-1.906	1
547		8	max	1041.871	3	1575.751	1	49.674	4	0	4	.054	4	1.592	3
548			min	-759.133	2	-1212.458	3	0	1	0	1	0	1	-2.884	1
549		9	max	1059.362	3	117.681	2	173.735	4	0	1	0	1	1.838	3
550			min	-615.609	2	.48	15	0	1	0	1	-.203	4	-3.271	1
551		10	max	1059.794	3	116.098	2	175.195	4	0	1	0	1	1.775	3
552			min	-615.033	2	.002	15	0	1	0	1	-.095	5	-3.311	1
553		11	max	1060.226	3	114.515	2	176.655	4	0	1	.014	4	1.713	3
554			min	-614.457	2	-1.727	6	0	1	0	1	0	1	-3.349	1
555		12	max	1077.926	3	771.72	3	206.298	4	0	1	0	1	1.499	3
556			min	-470.98	2	-1702.959	1	0	1	0	4	-.33	4	-2.979	1
557		13	max	1078.358	3	770.533	3	207.758	4	0	1	0	1	1.021	3
558			min	-470.404	2	-1704.542	1	0	1	0	4	-.201	4	-1.921	1
559		14	max	1078.79	3	769.345	3	209.218	4	0	1	0	1	.543	3
560			min	-469.827	2	-1706.125	1	0	1	0	4	-.072	4	-.863	1
561		15	max	1079.222	3	768.158	3	210.678	4	0	1	.058	4	.248	2
562			min	-469.251	2	-1707.708	1	0	1	0	4	0	1	0	15
563		16	max	1079.654	3	766.97	3	212.139	4	0	1	.19	4	1.257	1
564			min	-468.675	2	-1709.291	1	0	1	0	4	0	1	-.411	3
565		17	max	1080.086	3	765.783	3	213.599	4	0	1	.322	4	2.318	1
566			min	-468.099	2	-1710.875	1	0	1	0	4	0	1	-.886	3
567		18	max	-12.688	12	1879.209	1	0	1	0	4	.314	4	1.191	1
568			min	-372.303	1	-711.551	3	-35.51	5	0	1	0	1	-.461	3
569		19	max	-12.4	12	1877.626	1	0	1	0	4	.293	4	.025	1
570			min	-371.727	1	-712.738	3	-34.05	5	0	1	0	1	-.019	3
571	M9	1	max	168.67	1	532.823	3	80.8	4	0	3	-.011	12	0	15
572			min	6.843	12	-475.27	1	3.881	12	0	4	-.214	1	-.014	1
573		2	max	169.246	1	531.636	3	82.26	4	0	3	-.008	12	.281	1
574			min	7.131	12	-476.853	1	3.881	12	0	4	-.167	1	-.332	3
575		3	max	330.584	3	556.181	1	75.945	1	0	1	-.006	12	.566	1
576			min	-224.245	2	-393.223	3	3.835	12	0	3	-.119	1	-.652	3
577		4	max	331.016	3	554.598	1	75.945	1	0	1	-.004	12	.221	1
578			min	-223.669	2	-394.41	3	3.835	12	0	3	-.072	1	-.407	3
579		5	max	331.448	3	553.015	1	75.945	1	0	1	-.001	12	-.005	15
580			min	-223.093	2	-395.597	3	3.835	12	0	3	-.032	4	-.162	3
581		6	max	331.88	3	551.432	1	75.945	1	0	1	.022	1	.084	3
582			min	-222.517	2	-396.785	3	3.835	12	0	3	-.018	5	-.465	1
583		7	max	332.313	3	549.848	1	75.945	1	0	1	.069	1	.331	3
584			min	-221.941	2	-397.972	3	3.835	12	0	3	-.009	5	-.807	1
585		8	max	332.745	3	548.265	1	75.945	1	0	1	.116	1	.578	3
586			min	-221.364	2	-399.16	3	3.835	12	0	3	0	15	-1.147	1
587		9	max	343.924	3	35.288	2	120.49	1	0	3	-.004	12	.676	3
588			min	-152.515	2	.488	15	5.868	12	0	9	-.167	4	-1.306	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	344.356	3	33.705	2	120.49	1	0	3	.001	1	.659	3
590		min	-151.939	2	.011	15	5.868	12	0	9	-.109	4	-1.318	1
591	11	max	344.788	3	32.121	2	120.49	1	0	3	.076	1	.643	3
592		min	-151.363	2	-1.869	6	5.868	12	0	9	-.066	5	-1.329	1
593	12	max	355.863	3	260.249	3	177.323	4	0	3	-.005	12	.561	3
594		min	-84.391	10	-586.552	1	3.43	12	0	1	-.274	4	-1.174	1
595	13	max	356.296	3	259.062	3	178.783	4	0	3	-.003	12	.4	3
596		min	-83.911	10	-588.135	1	3.43	12	0	1	-.163	4	-.809	1
597	14	max	356.728	3	257.874	3	180.243	4	0	3	-.001	12	.24	3
598		min	-83.431	10	-589.718	1	3.43	12	0	1	-.052	4	-.444	1
599	15	max	357.16	3	256.687	3	181.703	4	0	3	.06	4	.08	3
600		min	-82.951	10	-591.302	1	3.43	12	0	1	0	12	-.077	1
601	16	max	357.592	3	255.5	3	183.163	4	0	3	.174	4	.29	2
602		min	-82.47	10	-592.885	1	3.43	12	0	1	.003	12	-.079	3
603	17	max	358.024	3	254.312	3	184.623	4	0	3	.288	4	.659	1
604		min	-81.99	10	-594.468	1	3.43	12	0	1	.005	12	-.237	3
605	18	max	-6.939	12	550.643	1	85.295	1	0	1	.259	4	.329	1
606		min	-169.69	1	-202.821	3	-86.222	5	0	3	.008	12	-.117	3
607	19	max	-6.651	12	549.06	1	85.295	1	0	1	.217	1	.01	3
608		min	-169.114	1	-204.009	3	-84.762	5	0	3	.01	12	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.193	1	.007	3	1.29e-2	1	NC	1	NC	1
2			min	-727	4	-.032	3	-.003	2	-2.024e-3	3	NC	1	NC	1
3		2	max	0	1	.153	3	.031	1	1.417e-2	1	NC	5	NC	2
4			min	-727	4	.003	15	-.017	5	-1.872e-3	3	1162.051	3	7129.355	1
5		3	max	0	1	.304	3	.073	1	1.544e-2	1	NC	5	NC	3
6			min	-727	4	-.053	1	-.021	5	-1.72e-3	3	641.39	3	3018.857	1
7		4	max	0	1	.397	3	.107	1	1.672e-2	1	NC	5	NC	3
8			min	-727	4	-.108	1	-.016	5	-1.569e-3	3	503.323	3	2032.006	1
9		5	max	0	1	.42	3	.124	1	1.799e-2	1	NC	5	NC	3
10			min	-727	4	-.102	1	-.006	5	-1.417e-3	3	477.875	3	1750.8	1
11		6	max	0	1	.374	3	.119	1	1.926e-2	1	NC	5	NC	3
12			min	-727	4	-.037	1	.004	15	-1.265e-3	3	530.949	3	1832.248	1
13		7	max	0	1	.275	3	.092	1	2.053e-2	1	NC	5	NC	3
14			min	-727	4	.003	15	.003	10	-1.114e-3	3	702.207	3	2367.038	1
15		8	max	0	1	.204	1	.052	1	2.181e-2	1	NC	1	NC	2
16			min	-727	4	.006	15	-.002	10	-9.622e-4	3	1194.322	3	4221.718	1
17		9	max	0	1	.319	1	.021	3	2.308e-2	1	NC	5	NC	1
18			min	-727	4	.009	15	-.006	10	-8.105e-4	3	1706.33	1	NC	1
19		10	max	0	1	.371	1	.02	3	2.435e-2	1	NC	3	NC	1
20			min	-727	4	-.019	3	-.014	2	-6.589e-4	3	1215.001	1	NC	1
21		11	max	0	12	.319	1	.021	3	2.308e-2	1	NC	5	NC	1
22			min	-727	4	.009	15	-.013	5	-8.105e-4	3	1706.33	1	NC	1
23		12	max	0	12	.204	1	.052	1	2.181e-2	1	NC	1	NC	2
24			min	-727	4	.006	15	-.013	5	-9.622e-4	3	1194.322	3	4221.718	1
25		13	max	0	12	.275	3	.092	1	2.053e-2	1	NC	5	NC	3
26			min	-727	4	.002	15	-.005	5	-1.114e-3	3	702.207	3	2367.038	1
27		14	max	0	12	.374	3	.119	1	1.926e-2	1	NC	5	NC	3
28			min	-727	4	-.037	1	.005	15	-1.265e-3	3	530.949	3	1832.248	1
29		15	max	0	12	.42	3	.124	1	1.799e-2	1	NC	5	NC	3
30			min	-727	4	-.102	1	.007	10	-1.417e-3	3	477.875	3	1750.8	1
31		16	max	0	12	.397	3	.107	1	1.672e-2	1	NC	5	NC	3
32			min	-727	4	-.108	1	.007	10	-1.569e-3	3	503.323	3	2032.006	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.304	3	.073	1	1.544e-2	1	NC	5	NC	3
34		min	-7.727	4	-.053	1	.004	10	-1.72e-3	3	641.39	3	3018.857	1
35	18	max	0	12	.153	3	.031	1	1.417e-2	1	NC	5	NC	2
36		min	-7.727	4	.002	15	0	10	-1.872e-3	3	1162.051	3	7129.355	1
37	19	max	0	12	.193	1	.007	3	1.29e-2	1	NC	1	NC	1
38		min	-7.727	4	-.032	3	-.003	2	-2.024e-3	3	NC	1	NC	1
39	M14	1	max	0	.276	3	.006	3	7.746e-3	1	NC	1	NC	1
40		min	-.551	4	-.593	1	-.003	2	-4.264e-3	3	NC	1	NC	1
41	2	max	0	1	.488	3	.021	1	9.043e-3	1	NC	5	NC	1
42		min	-.551	4	-.899	1	-.025	5	-5.068e-3	3	704.823	1	9095.888	5
43	3	max	0	1	.672	3	.056	1	1.034e-2	1	NC	15	NC	2
44		min	-.551	4	-1.171	1	-.031	5	-5.872e-3	3	373.998	1	3936.24	1
45	4	max	0	1	.808	3	.089	1	1.164e-2	1	NC	15	NC	3
46		min	-.551	4	-1.381	1	-.022	5	-6.676e-3	3	273.987	1	2453.401	1
47	5	max	0	1	.885	3	.108	1	1.294e-2	1	8959.341	15	NC	3
48		min	-.551	4	-1.518	1	-.005	5	-7.48e-3	3	233.399	1	2025.446	1
49	6	max	0	1	.905	3	.106	1	1.423e-2	1	8431.965	15	NC	3
50		min	-.551	4	-1.58	1	.005	10	-8.285e-3	3	218.888	1	2061.958	1
51	7	max	0	1	.874	3	.084	1	1.553e-2	1	8513.678	15	NC	3
52		min	-.551	4	-1.575	1	.002	10	-9.089e-3	3	219.995	1	2610.638	1
53	8	max	0	1	.812	3	.049	1	1.683e-2	1	9024.689	15	NC	2
54		min	-.551	4	-1.524	1	-.002	10	-9.893e-3	3	231.905	1	4430.574	4
55	9	max	0	1	.746	3	.032	4	1.812e-2	1	9740.108	15	NC	1
56		min	-.551	4	-1.461	1	-.006	10	-1.07e-2	3	248.898	1	6550.764	4
57	10	max	0	1	.715	3	.018	3	1.942e-2	1	NC	15	NC	1
58		min	-.551	4	-1.428	1	-.012	2	-1.15e-2	3	258.743	1	NC	1
59	11	max	0	12	.746	3	.018	3	1.812e-2	1	9740.077	15	NC	1
60		min	-.551	4	-1.461	1	-.025	5	-1.07e-2	3	248.898	1	9075.895	5
61	12	max	0	12	.812	3	.049	1	1.683e-2	1	9024.595	15	NC	2
62		min	-.551	4	-1.524	1	-.03	5	-9.893e-3	3	231.905	1	4571.851	1
63	13	max	0	12	.874	3	.084	1	1.553e-2	1	8513.515	15	NC	3
64		min	-.551	4	-1.575	1	-.02	5	-9.089e-3	3	219.995	1	2610.638	1
65	14	max	0	12	.905	3	.106	1	1.423e-2	1	8431.728	15	NC	3
66		min	-.551	4	-1.58	1	-.002	5	-8.285e-3	3	218.888	1	2061.958	1
67	15	max	0	12	.885	3	.108	1	1.294e-2	1	8959.005	15	NC	3
68		min	-.551	4	-1.518	1	.006	10	-7.48e-3	3	233.399	1	2025.446	1
69	16	max	0	12	.808	3	.089	1	1.164e-2	1	NC	15	NC	3
70		min	-.551	4	-1.381	1	.005	10	-6.676e-3	3	273.987	1	2453.401	1
71	17	max	0	12	.672	3	.056	1	1.034e-2	1	NC	15	NC	2
72		min	-.551	4	-1.171	1	.003	10	-5.872e-3	3	373.998	1	3936.24	1
73	18	max	0	12	.488	3	.033	4	9.043e-3	1	NC	5	NC	1
74		min	-.551	4	-.899	1	0	10	-5.068e-3	3	704.823	1	6321.922	4
75	19	max	0	12	.276	3	.006	3	7.746e-3	1	NC	1	NC	1
76		min	-.551	4	-.593	1	-.003	2	-4.264e-3	3	NC	1	NC	1
77	M15	1	max	0	.283	3	.005	3	3.56e-3	3	NC	1	NC	1
78		min	-.448	4	-.592	1	-.003	2	-7.887e-3	1	NC	1	NC	1
79	2	max	0	12	.43	3	.021	1	4.227e-3	3	NC	5	NC	1
80		min	-.448	4	-.922	1	-.037	5	-9.217e-3	1	654.399	1	6083.432	5
81	3	max	0	12	.562	3	.056	1	4.894e-3	3	NC	15	NC	2
82		min	-.448	4	-1.212	1	-.046	5	-1.055e-2	1	348.248	1	3914.574	1
83	4	max	0	12	.667	3	.09	1	5.561e-3	3	NC	15	NC	3
84		min	-.448	4	-1.435	1	-.034	5	-1.188e-2	1	256.353	1	2442.542	1
85	5	max	0	12	.738	3	.108	1	6.228e-3	3	8973.129	15	NC	3
86		min	-.448	4	-1.574	1	-.011	5	-1.321e-2	1	219.908	1	2017.011	1
87	6	max	0	12	.776	3	.107	1	6.895e-3	3	8446.649	15	NC	3
88		min	-.448	4	-1.63	1	.006	10	-1.454e-2	1	208.221	1	2052.662	1
89	7	max	0	12	.783	3	.085	1	7.562e-3	3	8530.76	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.448	4	-1.611	1	.003	10	-1.587e-2	1	211.926	1	2595.452	1
91	8	max	0	12	.768	3	.06	4	8.229e-3	3	9045.701	15	NC	2
92		min	-.449	4	-1.544	1	-.001	10	-1.72e-2	1	226.86	1	3573.206	4
93	9	max	0	12	.745	3	.042	4	8.895e-3	3	9765.94	15	NC	1
94		min	-.449	4	-1.466	1	-.005	10	-1.852e-2	1	247.297	1	5056.653	4
95	10	max	0	1	.733	3	.016	3	9.562e-3	3	NC	15	NC	1
96		min	-.449	4	-1.426	1	-.011	2	-1.985e-2	1	259.119	1	NC	1
97	11	max	0	1	.745	3	.017	3	8.895e-3	3	9765.916	15	NC	1
98		min	-.449	4	-1.466	1	-.035	5	-1.852e-2	1	247.297	1	6417.854	5
99	12	max	0	1	.768	3	.049	1	8.229e-3	3	9045.633	15	NC	2
100		min	-.448	4	-1.544	1	-.041	5	-1.72e-2	1	226.86	1	4525.672	1
101	13	max	0	1	.783	3	.085	1	7.562e-3	3	8530.645	15	NC	3
102		min	-.448	4	-1.611	1	-.028	5	-1.587e-2	1	211.926	1	2595.452	1
103	14	max	0	1	.776	3	.107	1	6.895e-3	3	8446.485	15	NC	3
104		min	-.448	4	-1.63	1	-.003	5	-1.454e-2	1	208.221	1	2052.662	1
105	15	max	0	1	.738	3	.108	1	6.228e-3	3	8972.9	15	NC	3
106		min	-.448	4	-1.574	1	.007	10	-1.321e-2	1	219.908	1	2017.011	1
107	16	max	0	1	.667	3	.09	1	5.561e-3	3	NC	15	NC	3
108		min	-.448	4	-1.435	1	.005	10	-1.188e-2	1	256.353	1	2442.542	1
109	17	max	0	1	.562	3	.066	4	4.894e-3	3	NC	15	NC	2
110		min	-.448	4	-1.212	1	.003	10	-1.055e-2	1	348.248	1	3241.9	4
111	18	max	0	1	.43	3	.045	4	4.227e-3	3	NC	5	NC	1
112		min	-.448	4	-.922	1	0	10	-9.217e-3	1	654.399	1	4705.75	4
113	19	max	0	1	.283	3	.005	3	3.56e-3	3	NC	1	NC	1
114		min	-.448	4	-.592	1	-.003	2	-7.887e-3	1	NC	1	NC	1
115	M16	1	max	0	.187	1	.005	3	6.556e-3	3	NC	1	NC	1
116		min	-.143	4	-.098	3	-.002	2	-1.212e-2	1	NC	1	NC	1
117	2	max	0	12	.023	9	.031	1	7.402e-3	3	NC	5	NC	2
118		min	-.143	4	-.046	3	-.026	5	-1.321e-2	1	1315.942	1	7209.674	1
119	3	max	0	12	0	15	.072	1	8.248e-3	3	NC	5	NC	3
120		min	-.143	4	-.13	2	-.033	5	-1.43e-2	1	738.481	1	3034.32	1
121	4	max	0	12	.011	3	.107	1	9.094e-3	3	NC	5	NC	3
122		min	-.144	4	-.198	2	-.026	5	-1.539e-2	1	597.95	1	2034.945	1
123	5	max	0	12	.004	12	.125	1	9.94e-3	3	NC	5	NC	3
124		min	-.144	4	-.201	2	-.011	5	-1.648e-2	1	600.655	1	1747.634	1
125	6	max	0	12	0	13	.12	1	1.079e-2	3	NC	5	NC	3
126		min	-.144	4	-.138	2	.004	15	-1.757e-2	1	736.08	2	1821.522	1
127	7	max	0	12	.03	9	.094	1	1.163e-2	3	NC	3	NC	3
128		min	-.144	4	-.083	3	.004	10	-1.866e-2	1	1193.801	2	2336.661	1
129	8	max	0	12	.166	1	.054	1	1.248e-2	3	NC	1	NC	2
130		min	-.144	4	-.143	3	0	10	-1.975e-2	1	4804.841	3	4091.558	1
131	9	max	0	12	.295	1	.027	4	1.332e-2	3	NC	5	NC	1
132		min	-.144	4	-.195	3	-.004	10	-2.084e-2	1	1986.893	1	7895.664	4
133	10	max	0	1	.353	1	.014	3	1.417e-2	3	NC	5	NC	1
134		min	-.144	4	-.218	3	-.01	2	-2.193e-2	1	1297.245	1	NC	1
135	11	max	0	1	.295	1	.015	3	1.332e-2	3	NC	5	NC	1
136		min	-.144	4	-.195	3	-.02	5	-2.084e-2	1	1986.893	1	NC	1
137	12	max	0	1	.166	1	.054	1	1.248e-2	3	NC	1	NC	2
138		min	-.143	4	-.143	3	-.021	5	-1.975e-2	1	4804.841	3	4091.558	1
139	13	max	0	1	.03	9	.094	1	1.163e-2	3	NC	3	NC	3
140		min	-.143	4	-.083	3	-.01	5	-1.866e-2	1	1193.801	2	2336.661	1
141	14	max	0	1	0	13	.12	1	1.079e-2	3	NC	5	NC	3
142		min	-.143	4	-.138	2	.005	15	-1.757e-2	1	736.08	2	1821.522	1
143	15	max	0	1	.004	12	.125	1	9.94e-3	3	NC	5	NC	3
144		min	-.143	4	-.201	2	.009	10	-1.648e-2	1	600.655	1	1747.634	1
145	16	max	0	1	.011	3	.107	1	9.094e-3	3	NC	5	NC	3
146		min	-.143	4	-.198	2	.007	10	-1.539e-2	1	597.95	1	2034.945	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	0	15	.072	1	8.248e-3	3	NC	5	NC	3
148			min	-.143	4	-.13	2	.005	10	-1.43e-2	1	738.481	1	3034.32	1
149		18	max	0	1	.023	9	.037	4	7.402e-3	3	NC	5	NC	2
150			min	-.143	4	-.046	3	0	10	-1.321e-2	1	1315.942	1	5760.585	4
151		19	max	0	1	.187	1	.005	3	6.556e-3	3	NC	1	NC	1
152			min	-.143	4	-.098	3	-.002	2	-1.212e-2	1	NC	1	NC	1
153	M2	1	max	.006	1	.005	2	.01	1	2.538e-3	5	NC	1	NC	2
154			min	-.006	3	-.009	3	-.683	4	-2.246e-4	1	NC	1	88.728	4
155		2	max	.006	1	.004	2	.009	1	2.546e-3	5	NC	1	NC	2
156			min	-.005	3	-.009	3	-.626	4	-2.104e-4	1	NC	1	96.695	4
157		3	max	.006	1	.004	2	.008	1	2.553e-3	5	NC	1	NC	2
158			min	-.005	3	-.009	3	-.57	4	-1.961e-4	1	NC	1	106.173	4
159		4	max	.005	1	.003	2	.008	1	2.56e-3	5	NC	1	NC	2
160			min	-.005	3	-.008	3	-.515	4	-1.819e-4	1	NC	1	117.558	4
161		5	max	.005	1	.002	2	.007	1	2.567e-3	5	NC	1	NC	2
162			min	-.004	3	-.008	3	-.461	4	-1.677e-4	1	NC	1	131.391	4
163		6	max	.005	1	.001	2	.006	1	2.575e-3	5	NC	1	NC	1
164			min	-.004	3	-.008	3	-.408	4	-1.535e-4	1	NC	1	148.426	4
165		7	max	.004	1	0	2	.005	1	2.584e-3	4	NC	1	NC	1
166			min	-.004	3	-.007	3	-.357	4	-1.392e-4	1	NC	1	169.731	4
167		8	max	.004	1	0	2	.004	1	2.595e-3	4	NC	1	NC	1
168			min	-.003	3	-.007	3	-.308	4	-1.25e-4	1	NC	1	196.874	4
169		9	max	.004	1	0	15	.004	1	2.606e-3	4	NC	1	NC	1
170			min	-.003	3	-.007	3	-.261	4	-1.108e-4	1	NC	1	232.222	4
171		10	max	.003	1	0	15	.003	1	2.617e-3	4	NC	1	NC	1
172			min	-.003	3	-.006	3	-.217	4	-9.653e-5	1	NC	1	279.496	4
173		11	max	.003	1	0	15	.003	1	2.628e-3	4	NC	1	NC	1
174			min	-.003	3	-.006	3	-.176	4	-8.229e-5	1	NC	1	344.818	4
175		12	max	.002	1	0	15	.002	1	2.639e-3	4	NC	1	NC	1
176			min	-.002	3	-.005	3	-.138	4	-6.806e-5	1	NC	1	438.855	4
177		13	max	.002	1	0	15	.001	1	2.65e-3	4	NC	1	NC	1
178			min	-.002	3	-.005	3	-.104	4	-5.383e-5	1	NC	1	581.592	4
179		14	max	.002	1	0	15	.001	1	2.661e-3	4	NC	1	NC	1
180			min	-.002	3	-.004	3	-.074	4	-3.96e-5	1	NC	1	814.188	4
181		15	max	.001	1	0	15	0	1	2.672e-3	4	NC	1	NC	1
182			min	-.001	3	-.004	3	-.049	4	-2.537e-5	1	NC	1	1233.136	4
183		16	max	.001	1	0	15	0	1	2.683e-3	4	NC	1	NC	1
184			min	0	3	-.003	3	-.029	4	-1.114e-5	1	NC	1	2111.894	4
185		17	max	0	1	0	15	0	1	2.694e-3	4	NC	1	NC	1
186			min	0	3	-.002	6	-.013	4	-3.291e-7	3	NC	1	4506.999	4
187		18	max	0	1	0	15	0	1	2.705e-3	4	NC	1	NC	1
188			min	0	3	-.001	6	-.004	4	5.474e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.716e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.282e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-4.104e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-5.239e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.015	4	1.806e-4	4	NC	1	NC	1
194			min	0	2	-.002	6	0	12	8.913e-7	12	NC	1	NC	1
195		3	max	0	3	-.001	15	.03	4	8.851e-4	4	NC	1	NC	1
196			min	0	2	-.005	6	0	12	2.193e-6	12	NC	1	NC	1
197		4	max	0	3	-.002	15	.044	4	1.59e-3	4	NC	1	NC	1
198			min	0	2	-.008	6	0	12	3.495e-6	12	NC	1	NC	1
199		5	max	.001	3	-.002	15	.057	4	2.294e-3	4	NC	1	NC	1
200			min	0	2	-.011	6	0	12	4.796e-6	12	9469.175	6	8933.566	5
201		6	max	.001	3	-.003	15	.071	4	2.999e-3	4	NC	1	NC	1
202			min	-.001	2	-.014	6	0	12	6.098e-6	12	7584.45	6	7816.494	5
203		7	max	.002	3	-.004	15	.083	4	3.703e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.001	2	-.016	6	0	12	7.4e-6	12	6454.681	6	7218.288	5
205		8	max	.002	3	-.004	15	.095	4	4.408e-3	4	NC	5	NC	1
206			min	-.001	2	-.018	6	0	12	8.702e-6	12	5757.275	6	6957.64	5
207		9	max	.002	3	-.004	15	.107	4	5.112e-3	4	NC	5	NC	1
208			min	-.002	2	-.019	6	0	12	1.e-5	12	5340.857	6	6961.465	5
209		10	max	.003	3	-.004	15	.118	4	5.817e-3	4	NC	5	NC	1
210			min	-.002	2	-.02	6	0	12	1.131e-5	12	5131.57	6	7214.749	5
211		11	max	.003	3	-.004	15	.128	4	6.521e-3	4	NC	5	NC	1
212			min	-.002	2	-.02	6	0	12	1.261e-5	12	5098.089	6	7748.077	5
213		12	max	.003	3	-.004	15	.139	4	7.225e-3	4	NC	5	NC	1
214			min	-.002	2	-.02	6	0	12	1.391e-5	12	5239.533	6	8646.525	5
215		13	max	.003	3	-.004	15	.148	4	7.93e-3	4	NC	5	NC	1
216			min	-.003	2	-.018	6	0	12	1.521e-5	12	5586.601	6	NC	1
217		14	max	.004	3	-.004	15	.158	4	8.634e-3	4	NC	5	NC	1
218			min	-.003	2	-.017	6	0	12	1.651e-5	12	6217.903	6	NC	1
219		15	max	.004	3	-.003	15	.168	4	9.339e-3	4	NC	3	NC	1
220			min	-.003	2	-.014	6	0	12	1.781e-5	12	7309.298	6	NC	1
221		16	max	.004	3	-.002	15	.177	4	1.004e-2	4	NC	1	NC	1
222			min	-.003	2	-.011	6	0	12	1.912e-5	12	9287.94	6	NC	1
223		17	max	.005	3	-.002	15	.187	4	1.075e-2	4	NC	1	NC	1
224			min	-.003	2	-.008	1	0	12	2.042e-5	12	NC	1	NC	1
225		18	max	.005	3	0	15	.197	4	1.145e-2	4	NC	1	NC	1
226			min	-.004	2	-.006	1	0	12	2.172e-5	12	NC	1	NC	1
227		19	max	.005	3	0	5	.208	4	1.216e-2	4	NC	1	NC	1
228			min	-.004	2	-.003	1	0	12	2.302e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.003	2	0	12	7.75e-5	1	NC	1	NC	3
230			min	0	3	-.005	3	-.208	4	-1.06e-3	4	NC	1	119.197	4
231		2	max	.003	1	.003	2	0	12	7.75e-5	1	NC	1	NC	3
232			min	0	3	-.005	3	-.191	4	-1.06e-3	4	NC	1	129.797	4
233		3	max	.003	1	.003	2	0	12	7.75e-5	1	NC	1	NC	3
234			min	0	3	-.005	3	-.174	4	-1.06e-3	4	NC	1	142.402	4
235		4	max	.002	1	.003	2	0	12	7.75e-5	1	NC	1	NC	3
236			min	0	3	-.004	3	-.157	4	-1.06e-3	4	NC	1	157.534	4
237		5	max	.002	1	.003	2	0	12	7.75e-5	1	NC	1	NC	2
238			min	0	3	-.004	3	-.141	4	-1.06e-3	4	NC	1	175.904	4
239		6	max	.002	1	.002	2	0	12	7.75e-5	1	NC	1	NC	2
240			min	0	3	-.004	3	-.125	4	-1.06e-3	4	NC	1	198.496	4
241		7	max	.002	1	.002	2	0	12	7.75e-5	1	NC	1	NC	2
242			min	0	3	-.003	3	-.109	4	-1.06e-3	4	NC	1	226.708	4
243		8	max	.002	1	.002	2	0	12	7.75e-5	1	NC	1	NC	2
244			min	0	3	-.003	3	-.094	4	-1.06e-3	4	NC	1	262.579	4
245		9	max	.002	1	.002	2	0	12	7.75e-5	1	NC	1	NC	2
246			min	0	3	-.003	3	-.08	4	-1.06e-3	4	NC	1	309.179	4
247		10	max	.001	1	.002	2	0	12	7.75e-5	1	NC	1	NC	2
248			min	0	3	-.003	3	-.067	4	-1.06e-3	4	NC	1	371.309	4
249		11	max	.001	1	.001	2	0	12	7.75e-5	1	NC	1	NC	1
250			min	0	3	-.002	3	-.054	4	-1.06e-3	4	NC	1	456.82	4
251		12	max	.001	1	.001	2	0	12	7.75e-5	1	NC	1	NC	1
252			min	0	3	-.002	3	-.043	4	-1.06e-3	4	NC	1	579.3	4
253		13	max	0	1	.001	2	0	12	7.75e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.032	4	-1.06e-3	4	NC	1	763.973	4
255		14	max	0	1	0	2	0	12	7.75e-5	1	NC	1	NC	1
256			min	0	3	-.001	3	-.023	4	-1.06e-3	4	NC	1	1062.166	4
257		15	max	0	1	0	2	0	12	7.75e-5	1	NC	1	NC	1
258			min	0	3	-.001	3	-.016	4	-1.06e-3	4	NC	1	1592.228	4
259		16	max	0	1	0	2	0	12	7.75e-5	1	NC	1	NC	1
260			min	0	3	0	3	-.009	4	-1.06e-3	4	NC	1	2681.428	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	7.75e-5	1	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-1.06e-3	4	NC	1	5543.77	4
263		18	max	0	1	0	2	0	12	7.75e-5	1	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-1.06e-3	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	7.75e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.06e-3	4	NC	1	NC	1
267	M6	1	max	.02	1	.02	2	0	1	2.654e-3	4	NC	3	NC	1
268			min	-.018	3	-.027	3	-.69	4	0	1	2984.363	2	87.821	4
269		2	max	.019	1	.018	2	0	1	2.658e-3	4	NC	3	NC	1
270			min	-.017	3	-.026	3	-.633	4	0	1	3284.413	2	95.708	4
271		3	max	.018	1	.017	2	0	1	2.662e-3	4	NC	3	NC	1
272			min	-.016	3	-.025	3	-.576	4	0	1	3648.54	2	105.089	4
273		4	max	.017	1	.015	2	0	1	2.665e-3	4	NC	3	NC	1
274			min	-.015	3	-.023	3	-.52	4	0	1	4095.908	2	116.359	4
275		5	max	.016	1	.013	2	0	1	2.669e-3	4	NC	3	NC	1
276			min	-.014	3	-.022	3	-.466	4	0	1	4653.671	2	130.052	4
277		6	max	.014	1	.011	2	0	1	2.673e-3	4	NC	3	NC	1
278			min	-.013	3	-.02	3	-.412	4	0	1	5361.362	2	146.913	4
279		7	max	.013	1	.01	2	0	1	2.677e-3	4	NC	1	NC	1
280			min	-.012	3	-.019	3	-.36	4	0	1	6278.413	2	168.003	4
281		8	max	.012	1	.008	2	0	1	2.68e-3	4	NC	1	NC	1
282			min	-.011	3	-.017	3	-.311	4	0	1	7497.673	2	194.871	4
283		9	max	.011	1	.007	2	0	1	2.684e-3	4	NC	1	NC	1
284			min	-.01	3	-.016	3	-.263	4	0	1	9171.131	2	229.863	4
285		10	max	.01	1	.005	2	0	1	2.688e-3	4	NC	1	NC	1
286			min	-.009	3	-.014	3	-.219	4	0	1	NC	1	276.66	4
287		11	max	.009	1	.004	2	0	1	2.692e-3	4	NC	1	NC	1
288			min	-.008	3	-.013	3	-.177	4	0	1	NC	1	341.323	4
289		12	max	.008	1	.003	2	0	1	2.695e-3	4	NC	1	NC	1
290			min	-.007	3	-.011	3	-.139	4	0	1	NC	1	434.413	4
291		13	max	.007	1	.002	2	0	1	2.699e-3	4	NC	1	NC	1
292			min	-.006	3	-.01	3	-.105	4	0	1	NC	1	575.714	4
293		14	max	.006	1	.001	2	0	1	2.703e-3	4	NC	1	NC	1
294			min	-.005	3	-.008	3	-.075	4	0	1	NC	1	805.976	4
295		15	max	.004	1	0	2	0	1	2.707e-3	4	NC	1	NC	1
296			min	-.004	3	-.007	3	-.05	4	0	1	NC	1	1220.731	4
297		16	max	.003	1	0	2	0	1	2.71e-3	4	NC	1	NC	1
298			min	-.003	3	-.005	3	-.029	4	0	1	NC	1	2090.727	4
299		17	max	.002	1	0	2	0	1	2.714e-3	4	NC	1	NC	1
300			min	-.002	3	-.003	3	-.014	4	0	1	NC	1	4462.112	4
301		18	max	.001	1	0	2	0	1	2.718e-3	4	NC	1	NC	1
302			min	-.001	3	-.002	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.721e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.238e-4	4	NC	1	NC	1
307		2	max	0	3	0	15	.015	4	1.607e-4	4	NC	1	NC	1
308			min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
309		3	max	.002	3	-.001	15	.03	4	8.452e-4	4	NC	1	NC	1
310			min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
311		4	max	.003	3	-.002	15	.044	4	1.53e-3	4	NC	1	NC	1
312			min	-.002	2	-.009	3	0	1	0	1	NC	1	9881.833	4
313		5	max	.003	3	-.003	15	.058	4	2.214e-3	4	NC	1	NC	1
314			min	-.003	2	-.011	3	0	1	0	1	9537.557	4	7969.381	4
315		6	max	.004	3	-.003	15	.071	4	2.899e-3	4	NC	1	NC	1
316			min	-.004	2	-.014	4	0	1	0	1	7633.744	4	6932.505	4
317		7	max	.005	3	-.004	15	.083	4	3.583e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318			min	-.005	2	-.016	4	0	1	0	1	6492.93	4	6358.687	4
319		8	max	.006	3	-.004	15	.095	4	4.268e-3	4	NC	2	NC	1
320			min	-.006	2	-.018	4	0	1	0	1	5788.72	4	6080.532	4
321		9	max	.007	3	-.005	15	.106	4	4.952e-3	4	NC	5	NC	1
322			min	-.006	2	-.02	4	0	1	0	1	5367.992	4	6027.033	4
323		10	max	.008	3	-.005	15	.117	4	5.637e-3	4	NC	5	NC	1
324			min	-.007	2	-.021	4	0	1	0	1	5156.019	4	6176.853	4
325		11	max	.009	3	-.005	15	.127	4	6.321e-3	4	NC	5	NC	1
326			min	-.008	2	-.021	4	0	1	0	1	5121.034	4	6544.606	4
327		12	max	.009	3	-.005	15	.137	4	7.006e-3	4	NC	5	NC	1
328			min	-.009	2	-.02	4	0	1	0	1	5261.958	4	7183.89	4
329		13	max	.01	3	-.004	15	.147	4	7.69e-3	4	NC	5	NC	1
330			min	-.01	2	-.019	4	0	1	0	1	5609.482	4	8207.683	4
331		14	max	.011	3	-.004	15	.156	4	8.375e-3	4	NC	2	NC	1
332			min	-.011	2	-.017	4	0	1	0	1	6242.421	4	9842.225	4
333		15	max	.012	3	-.003	15	.165	4	9.059e-3	4	NC	1	NC	1
334			min	-.011	2	-.015	4	0	1	0	1	7337.215	4	NC	1
335		16	max	.013	3	-.003	15	.173	4	9.744e-3	4	NC	1	NC	1
336			min	-.012	2	-.012	4	0	1	0	1	9322.513	4	NC	1
337		17	max	.014	3	-.002	15	.182	4	1.043e-2	4	NC	1	NC	1
338			min	-.013	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.015	3	-.001	15	.191	4	1.111e-2	4	NC	1	NC	1
340			min	-.014	2	-.008	1	0	1	0	1	NC	1	NC	1
341		19	max	.016	3	0	15	.201	4	1.18e-2	4	NC	1	NC	1
342			min	-.015	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.013	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.016	3	-.201	4	-1.178e-3	4	NC	1	123.351	4
345		2	max	.007	1	.013	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.015	3	-.185	4	-1.178e-3	4	NC	1	134.332	4
347		3	max	.007	1	.012	2	0	1	0	1	NC	1	NC	1
348			min	-.001	3	-.014	3	-.168	4	-1.178e-3	4	NC	1	147.388	4
349		4	max	.007	1	.011	2	0	1	0	1	NC	1	NC	1
350			min	-.001	3	-.013	3	-.152	4	-1.178e-3	4	NC	1	163.063	4
351		5	max	.006	1	.01	2	0	1	0	1	NC	1	NC	1
352			min	-.001	3	-.012	3	-.136	4	-1.178e-3	4	NC	1	182.09	4
353		6	max	.006	1	.01	2	0	1	0	1	NC	1	NC	1
354			min	-.001	3	-.011	3	-.121	4	-1.178e-3	4	NC	1	205.49	4
355		7	max	.005	1	.009	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.01	3	-.106	4	-1.178e-3	4	NC	1	234.711	4
357		8	max	.005	1	.008	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.01	3	-.091	4	-1.178e-3	4	NC	1	271.865	4
359		9	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.009	3	-.077	4	-1.178e-3	4	NC	1	320.132	4
361		10	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.008	3	-.065	4	-1.178e-3	4	NC	1	384.483	4
363		11	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.007	3	-.052	4	-1.178e-3	4	NC	1	473.054	4
365		12	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.006	3	-.041	4	-1.178e-3	4	NC	1	599.918	4
367		13	max	.003	1	.004	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.005	3	-.031	4	-1.178e-3	4	NC	1	791.204	4
369		14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.004	3	-.023	4	-1.178e-3	4	NC	1	1100.081	4
371		15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.003	3	-.015	4	-1.178e-3	4	NC	1	1649.147	4
373		16	max	.001	1	.002	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	-.009	4	-1.178e-3	4	NC	1	2777.434	4





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.004	4	-1.178e-3	4	NC	1	5742.618	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-1.178e-3	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-1.178e-3	4	NC	1	NC	1
381	M10	1	max	.006	1	.005	2	0	12	2.636e-3	4	NC	1	NC	2
382			min	-.006	3	-.009	3	-.688	4	1.195e-5	12	NC	1	88.049	4
383		2	max	.006	1	.004	2	0	12	2.639e-3	4	NC	1	NC	2
384			min	-.005	3	-.009	3	-.631	4	1.121e-5	12	NC	1	95.955	4
385		3	max	.006	1	.004	2	0	12	2.643e-3	4	NC	1	NC	2
386			min	-.005	3	-.009	3	-.575	4	1.048e-5	12	NC	1	105.361	4
387		4	max	.005	1	.003	2	0	12	2.646e-3	4	NC	1	NC	2
388			min	-.005	3	-.008	3	-.519	4	9.742e-6	12	NC	1	116.66	4
389		5	max	.005	1	.002	2	0	12	2.65e-3	4	NC	1	NC	2
390			min	-.004	3	-.008	3	-.464	4	9.007e-6	12	NC	1	130.388	4
391		6	max	.005	1	.001	2	0	12	2.653e-3	4	NC	1	NC	1
392			min	-.004	3	-.008	3	-.411	4	8.272e-6	12	NC	1	147.293	4
393		7	max	.004	1	0	2	0	12	2.657e-3	4	NC	1	NC	1
394			min	-.004	3	-.007	3	-.36	4	7.537e-6	12	NC	1	168.437	4
395		8	max	.004	1	0	2	0	12	2.66e-3	4	NC	1	NC	1
396			min	-.003	3	-.007	3	-.31	4	6.802e-6	12	NC	1	195.375	4
397		9	max	.004	1	0	2	0	12	2.664e-3	4	NC	1	NC	1
398			min	-.003	3	-.007	3	-.263	4	6.067e-6	12	NC	1	230.457	4
399		10	max	.003	1	-.001	2	0	12	2.667e-3	4	NC	1	NC	1
400			min	-.003	3	-.006	3	-.218	4	5.332e-6	12	NC	1	277.375	4
401		11	max	.003	1	-.002	2	0	12	2.671e-3	4	NC	1	NC	1
402			min	-.003	3	-.006	3	-.177	4	4.597e-6	12	NC	1	342.206	4
403		12	max	.002	1	-.001	15	0	12	2.674e-3	4	NC	1	NC	1
404			min	-.002	3	-.005	3	-.139	4	3.862e-6	12	NC	1	435.538	4
405		13	max	.002	1	-.001	15	0	12	2.678e-3	4	NC	1	NC	1
406			min	-.002	3	-.005	4	-.105	4	3.127e-6	12	NC	1	577.207	4
407		14	max	.002	1	-.001	15	0	12	2.682e-3	4	NC	1	NC	1
408			min	-.002	3	-.005	4	-.075	4	2.392e-6	12	NC	1	808.071	4
409		15	max	.001	1	-.001	15	0	12	2.685e-3	4	NC	1	NC	1
410			min	-.001	3	-.004	4	-.049	4	1.657e-6	12	NC	1	1223.916	4
411		16	max	.001	1	0	15	0	12	2.689e-3	4	NC	1	NC	1
412			min	0	3	-.003	4	-.029	4	8.347e-7	10	NC	1	2096.217	4
413		17	max	0	1	0	15	0	12	2.692e-3	4	NC	1	NC	1
414			min	0	3	-.002	4	-.014	4	-3.096e-6	1	NC	1	4473.982	4
415		18	max	0	1	0	15	0	12	2.696e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.004	4	-1.733e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.699e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-3.156e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	9.62e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-5.187e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.015	4	1.706e-4	4	NC	1	NC	1
422			min	0	2	-.003	4	0	1	-1.903e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	.029	4	8.598e-4	4	NC	1	NC	1
424			min	0	2	-.006	4	0	1	-4.767e-5	1	NC	1	NC	1
425		4	max	0	3	-.002	15	.044	4	1.549e-3	4	NC	1	NC	1
426			min	0	2	-.009	4	0	1	-7.632e-5	1	NC	1	NC	1
427		5	max	.001	3	-.003	15	.057	4	2.238e-3	4	NC	1	NC	1
428			min	0	2	-.012	4	0	1	-1.05e-4	1	9036.891	4	8570.361	4
429		6	max	.001	3	-.004	15	.07	4	2.928e-3	4	NC	1	NC	1
430			min	-.001	2	-.014	4	-.001	1	-1.336e-4	1	7271.191	4	7486.056	4
431		7	max	.002	3	-.004	15	.083	4	3.617e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.001	2	-.017	4	-.001	1	-1.623e-4	1	6210.585	4	6899.013	4
433		8	max	.002	3	-.005	15	.094	4	4.306e-3	4	NC	5	NC	1
434			min	-.001	2	-.019	4	-.002	1	-1.909e-4	1	5555.921	4	6633.487	4
435		9	max	.002	3	-.005	15	.106	4	4.995e-3	4	NC	5	NC	1
436			min	-.002	2	-.02	4	-.002	1	-2.196e-4	1	5166.611	4	6617.332	4
437		10	max	.003	3	-.005	15	.117	4	5.685e-3	4	NC	5	NC	1
438			min	-.002	2	-.021	4	-.002	1	-2.482e-4	1	4974.2	4	6833.202	4
439		11	max	.003	3	-.005	15	.127	4	6.374e-3	4	NC	5	NC	1
440			min	-.002	2	-.021	4	-.003	1	-2.768e-4	1	4950.115	4	7305.594	4
441		12	max	.003	3	-.005	15	.137	4	7.063e-3	4	NC	5	NC	1
442			min	-.002	2	-.021	4	-.003	1	-3.055e-4	1	5094.676	4	8107.389	4
443		13	max	.003	3	-.005	15	.146	4	7.753e-3	4	NC	5	NC	1
444			min	-.003	2	-.02	4	-.004	1	-3.341e-4	1	5438.596	4	9388.906	4
445		14	max	.004	3	-.004	15	.156	4	8.442e-3	4	NC	5	NC	1
446			min	-.003	2	-.018	4	-.005	1	-3.628e-4	1	6059.123	4	NC	1
447		15	max	.004	3	-.004	15	.165	4	9.131e-3	4	NC	3	NC	1
448			min	-.003	2	-.015	4	-.005	1	-3.914e-4	1	7128.341	4	NC	1
449		16	max	.004	3	-.003	15	.174	4	9.82e-3	4	NC	1	NC	1
450			min	-.003	2	-.012	4	-.006	1	-4.201e-4	1	9063.678	4	NC	1
451		17	max	.005	3	-.002	15	.183	4	1.051e-2	4	NC	1	NC	1
452			min	-.003	2	-.009	4	-.007	1	-4.487e-4	1	NC	1	NC	1
453		18	max	.005	3	-.001	15	.193	4	1.12e-2	4	NC	1	NC	1
454			min	-.004	2	-.006	1	-.008	1	-4.774e-4	1	NC	1	NC	1
455		19	max	.005	3	0	10	.203	4	1.189e-2	4	NC	1	NC	1
456			min	-.004	2	-.003	1	-.009	1	-5.06e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.003	2	.009	1	-3.862e-6	12	NC	1	NC	3
458			min	0	3	-.005	3	-.203	4	-1.101e-3	4	NC	1	122.099	4
459		2	max	.003	1	.003	2	.009	1	-3.862e-6	12	NC	1	NC	3
460			min	0	3	-.005	3	-.187	4	-1.101e-3	4	NC	1	132.961	4
461		3	max	.003	1	.003	2	.008	1	-3.862e-6	12	NC	1	NC	3
462			min	0	3	-.005	3	-.17	4	-1.101e-3	4	NC	1	145.877	4
463		4	max	.002	1	.003	2	.007	1	-3.862e-6	12	NC	1	NC	3
464			min	0	3	-.004	3	-.154	4	-1.101e-3	4	NC	1	161.382	4
465		5	max	.002	1	.003	2	.006	1	-3.862e-6	12	NC	1	NC	2
466			min	0	3	-.004	3	-.138	4	-1.101e-3	4	NC	1	180.205	4
467		6	max	.002	1	.002	2	.006	1	-3.862e-6	12	NC	1	NC	2
468			min	0	3	-.004	3	-.122	4	-1.101e-3	4	NC	1	203.353	4
469		7	max	.002	1	.002	2	.005	1	-3.862e-6	12	NC	1	NC	2
470			min	0	3	-.003	3	-.107	4	-1.101e-3	4	NC	1	232.261	4
471		8	max	.002	1	.002	2	.004	1	-3.862e-6	12	NC	1	NC	2
472			min	0	3	-.003	3	-.092	4	-1.101e-3	4	NC	1	269.016	4
473		9	max	.002	1	.002	2	.004	1	-3.862e-6	12	NC	1	NC	2
474			min	0	3	-.003	3	-.078	4	-1.101e-3	4	NC	1	316.765	4
475		10	max	.001	1	.002	2	.003	1	-3.862e-6	12	NC	1	NC	2
476			min	0	3	-.003	3	-.065	4	-1.101e-3	4	NC	1	380.426	4
477		11	max	.001	1	.001	2	.002	1	-3.862e-6	12	NC	1	NC	1
478			min	0	3	-.002	3	-.053	4	-1.101e-3	4	NC	1	468.045	4
479		12	max	.001	1	.001	2	.002	1	-3.862e-6	12	NC	1	NC	1
480			min	0	3	-.002	3	-.042	4	-1.101e-3	4	NC	1	593.545	4
481		13	max	0	1	.001	2	.001	1	-3.862e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.032	4	-1.101e-3	4	NC	1	782.772	4
483		14	max	0	1	0	2	.001	1	-3.862e-6	12	NC	1	NC	1
484			min	0	3	-.001	3	-.023	4	-1.101e-3	4	NC	1	1088.32	4
485		15	max	0	1	0	2	0	1	-3.862e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.015	4	-1.101e-3	4	NC	1	1631.46	4
487		16	max	0	1	0	2	0	1	-3.862e-6	12	NC	1	NC	1
488			min	0	3	0	3	-.009	4	-1.101e-3	4	NC	1	2747.547	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	-3.862e-6	12	NC	1	NC	1
490		min	0	3	0	3	-.004	4	-1.101e-3	4	NC	1	5680.587	4
491	18	max	0	1	0	2	0	1	-3.862e-6	12	NC	1	NC	1
492		min	0	3	0	3	-.001	4	-1.101e-3	4	NC	1	NC	1
493	19	max	0	1	0	1	0	1	-3.862e-6	12	NC	1	NC	1
494		min	0	1	0	1	0	1	-1.101e-3	4	NC	1	NC	1
495	M1	1	max	.007	3	.193	.727	4	1.206e-2	1	NC	1	NC	1
496		min	-.003	2	-.032	3	0	12	-1.607e-2	3	NC	1	NC	1
497	2	max	.007	3	.096	1	.705	4	9.579e-3	4	NC	5	NC	1
498		min	-.003	2	-.016	3	-.007	1	-7.977e-3	3	1390.803	1	NC	1
499	3	max	.007	3	.009	3	.682	4	1.658e-2	4	NC	5	NC	1
500		min	-.003	2	-.008	1	-.01	1	-2.197e-4	1	668.391	1	6065.146	5
501	4	max	.006	3	.051	3	.659	4	1.444e-2	4	NC	15	NC	1
502		min	-.003	2	-.127	1	-.009	1	-3.423e-3	3	420.727	1	4280.455	5
503	5	max	.006	3	.104	3	.636	4	1.23e-2	4	9584.522	15	NC	1
504		min	-.003	2	-.251	1	-.006	1	-6.763e-3	3	302.735	1	3370.707	5
505	6	max	.006	3	.161	3	.612	4	1.44e-2	1	7578.187	15	NC	1
506		min	-.003	2	-.373	1	-.003	1	-1.01e-2	3	237.844	1	2820.695	5
507	7	max	.006	3	.216	3	.587	4	1.928e-2	1	6393.297	15	NC	1
508		min	-.003	2	-.481	1	0	3	-1.344e-2	3	199.61	1	2446.132	4
509	8	max	.006	3	.262	3	.562	4	2.415e-2	1	5692.032	15	NC	1
510		min	-.003	2	-.567	1	0	12	-1.678e-2	3	177.027	1	2173.82	4
511	9	max	.006	3	.292	3	.534	4	2.649e-2	1	5325.153	15	NC	1
512		min	-.003	2	-.621	1	0	1	-1.707e-2	3	165.263	1	2001.58	4
513	10	max	.006	3	.303	3	.504	4	2.713e-2	1	5213.045	15	NC	1
514		min	-.003	2	-.639	1	0	12	-1.531e-2	3	161.732	1	1950.755	4
515	11	max	.006	3	.296	3	.47	4	2.777e-2	1	5324.968	15	NC	1
516		min	-.003	2	-.621	1	0	12	-1.356e-2	3	165.466	1	1994.404	4
517	12	max	.005	3	.271	3	.433	4	2.611e-2	1	5691.591	15	NC	1
518		min	-.003	2	-.566	1	-.001	1	-1.158e-2	3	177.643	1	2141.226	4
519	13	max	.005	3	.231	3	.39	4	2.101e-2	1	6392.428	15	NC	1
520		min	-.003	2	-.478	1	0	1	-9.262e-3	3	201.106	1	2550.001	4
521	14	max	.005	3	.18	3	.344	4	1.591e-2	1	7576.583	15	NC	1
522		min	-.003	2	-.368	1	0	12	-6.947e-3	3	241.024	1	3458.069	4
523	15	max	.005	3	.121	3	.297	4	1.082e-2	1	9581.57	15	NC	1
524		min	-.002	2	-.245	1	0	12	-4.633e-3	3	309.222	1	5635.695	4
525	16	max	.005	3	.061	3	.251	4	1.001e-2	4	NC	15	NC	1
526		min	-.002	2	-.121	1	0	12	-2.318e-3	3	434.225	1	NC	1
527	17	max	.005	3	.003	3	.209	4	1.114e-2	4	NC	5	NC	1
528		min	-.002	2	-.005	2	0	12	-3.164e-6	3	698.411	1	NC	1
529	18	max	.005	3	.096	1	.173	4	7.345e-3	1	NC	5	NC	1
530		min	-.002	2	-.049	3	0	12	-2.267e-3	3	1466.467	1	NC	1
531	19	max	.005	3	.187	1	.143	4	1.428e-2	1	NC	1	NC	1
532		min	-.002	2	-.098	3	0	1	-4.613e-3	3	NC	1	NC	1
533	M5	1	max	.02	3	.371	.727	4	0	1	NC	1	NC	1
534		min	-.014	2	-.019	3	0	1	-7.597e-6	4	NC	1	NC	1
535	2	max	.02	3	.186	1	.71	4	8.486e-3	4	NC	5	NC	1
536		min	-.014	2	-.011	3	0	1	0	1	729.577	1	8489.725	4
537	3	max	.02	3	.027	3	.689	4	1.678e-2	4	NC	15	NC	1
538		min	-.014	2	-.028	1	0	1	0	1	338.251	1	4921.468	4
539	4	max	.02	3	.12	3	.666	4	1.367e-2	4	7176.125	15	NC	1
540		min	-.014	2	-.292	1	0	1	0	1	203.36	1	3728.13	4
541	5	max	.019	3	.253	3	.64	4	1.056e-2	4	4996.902	15	NC	1
542		min	-.013	2	-.584	1	0	1	0	1	141.005	1	3128.952	4
543	6	max	.019	3	.403	3	.613	4	7.456e-3	4	3832.701	15	NC	1
544		min	-.013	2	-.878	1	0	1	0	1	107.785	1	2751.61	4
545	7	max	.018	3	.551	3	.587	4	4.35e-3	4	3162.852	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.013	2	-1.146	1	0	1	0	1	88.711	1	2462.831	4
547		8	max	.018	3	.676	3	.561	4	1.243e-3	4	2774.578	15	NC	1
548			min	-.012	2	-1.362	1	0	1	0	1	77.672	1	2202.81	4
549		9	max	.018	3	.757	3	.535	4	1.721e-7	14	2575.678	15	NC	1
550			min	-.012	2	-1.498	1	0	1	-3.934e-6	5	72.027	1	1997.457	4
551		10	max	.017	3	.786	3	.503	4	2.855e-7	14	2515.725	15	NC	1
552			min	-.012	2	-1.544	1	0	1	-3.729e-6	5	70.348	1	1968.3	4
553		11	max	.017	3	.767	3	.469	4	3.988e-7	14	2575.769	15	NC	1
554			min	-.012	2	-1.498	1	0	1	-3.525e-6	5	72.128	1	2022.424	4
555		12	max	.016	3	.7	3	.434	4	7.938e-4	4	2774.795	15	NC	1
556			min	-.011	2	-1.359	1	0	1	0	1	78.008	1	2102.021	4
557		13	max	.016	3	.592	3	.392	4	2.777e-3	4	3163.295	15	NC	1
558			min	-.011	2	-1.137	1	0	1	0	1	89.593	1	2485.557	4
559		14	max	.015	3	.456	3	.344	4	4.761e-3	4	3833.566	15	NC	1
560			min	-.011	2	-.863	1	0	1	0	1	109.796	1	3516.958	4
561		15	max	.015	3	.304	3	.293	4	6.745e-3	4	4998.611	15	NC	1
562			min	-.011	2	-.565	1	0	1	0	1	145.428	1	6625.102	5
563		16	max	.015	3	.151	3	.245	4	8.728e-3	4	7179.713	15	NC	1
564			min	-.011	2	-.272	1	0	1	0	1	213.441	1	NC	1
565		17	max	.014	3	.009	3	.202	4	1.071e-2	4	NC	15	NC	1
566			min	-.01	2	-.014	2	0	1	0	1	363.218	1	NC	1
567		18	max	.014	3	.186	1	.168	4	5.419e-3	4	NC	5	NC	1
568			min	-.01	2	-.111	3	0	1	0	1	797.729	1	NC	1
569		19	max	.014	3	.353	1	.144	4	0	1	NC	1	NC	1
570			min	-.01	2	-.218	3	0	1	-3.659e-6	4	NC	1	NC	1
571	M9	1	max	.007	3	.193	1	.727	4	1.607e-2	3	NC	1	NC	1
572			min	-.003	2	-.032	3	0	1	-1.206e-2	1	NC	1	NC	1
573		2	max	.007	3	.096	1	.709	4	8.009e-3	5	NC	5	NC	1
574			min	-.003	2	-.016	3	0	12	-5.823e-3	1	1390.803	1	9186.789	4
575		3	max	.007	3	.009	3	.687	4	1.671e-2	4	NC	5	NC	1
576			min	-.003	2	-.008	1	0	12	-6.84e-6	10	668.391	1	5220.028	4
577		4	max	.006	3	.051	3	.664	4	1.311e-2	5	NC	15	NC	1
578			min	-.003	2	-.127	1	0	12	-4.655e-3	1	420.727	1	3867.147	4
579		5	max	.006	3	.104	3	.639	4	9.887e-3	5	9549.065	15	NC	1
580			min	-.003	2	-.251	1	0	12	-9.53e-3	1	302.735	1	3179.344	4
581		6	max	.006	3	.161	3	.613	4	1.01e-2	3	7551.258	15	NC	1
582			min	-.003	2	-.373	1	0	12	-1.44e-2	1	237.844	1	2751.8	4
583		7	max	.006	3	.216	3	.587	4	1.344e-2	3	6371.263	15	NC	1
584			min	-.003	2	-.481	1	0	1	-1.928e-2	1	199.61	1	2442.706	4
585		8	max	.006	3	.262	3	.561	4	1.678e-2	3	5672.827	15	NC	1
586			min	-.003	2	-.567	1	0	1	-2.415e-2	1	177.027	1	2188.763	4
587		9	max	.006	3	.292	3	.535	4	1.707e-2	3	5307.39	15	NC	1
588			min	-.003	2	-.621	1	0	12	-2.649e-2	1	165.263	1	1995.858	4
589		10	max	.006	3	.303	3	.504	4	1.531e-2	3	5195.698	15	NC	1
590			min	-.003	2	-.639	1	0	1	-2.713e-2	1	161.732	1	1951.644	4
591		11	max	.006	3	.296	3	.47	4	1.356e-2	3	5307.19	15	NC	1
592			min	-.003	2	-.621	1	0	1	-2.777e-2	1	165.466	1	2001.444	4
593		12	max	.005	3	.271	3	.433	4	1.158e-2	3	5672.454	15	NC	1
594			min	-.003	2	-.566	1	0	12	-2.611e-2	1	177.643	1	2126.559	4
595		13	max	.005	3	.231	3	.39	4	9.262e-3	3	6370.706	15	NC	1
596			min	-.003	2	-.478	1	0	12	-2.101e-2	1	201.106	1	2549	4
597		14	max	.005	3	.18	3	.343	4	6.947e-3	3	7550.45	15	NC	1
598			min	-.003	2	-.368	1	-.002	1	-1.591e-2	1	241.024	1	3537.51	5
599		15	max	.005	3	.121	3	.294	4	6.381e-3	5	9547.858	15	NC	1
600			min	-.002	2	-.245	1	-.006	1	-1.082e-2	1	309.222	1	6111.485	5
601		16	max	.005	3	.061	3	.246	4	8.569e-3	5	NC	15	NC	1
602			min	-.002	2	-.121	1	-.009	1	-5.721e-3	1	434.225	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.005	3	.003	3	.204	4	1.083e-2	4	NC	5	NC	1
604		min	-.002	2	-.005	2	-.009	1	-6.247e-4	1	698.411	1	NC	1
605	18	max	.005	3	.096	1	.17	4	5.196e-3	5	NC	5	NC	1
606		min	-.002	2	-.049	3	-.007	1	-7.345e-3	1	1466.467	1	NC	1
607	19	max	.005	3	.187	1	.143	4	4.613e-3	3	NC	1	NC	1
608		min	-.002	2	-.098	3	0	12	-1.428e-2	1	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

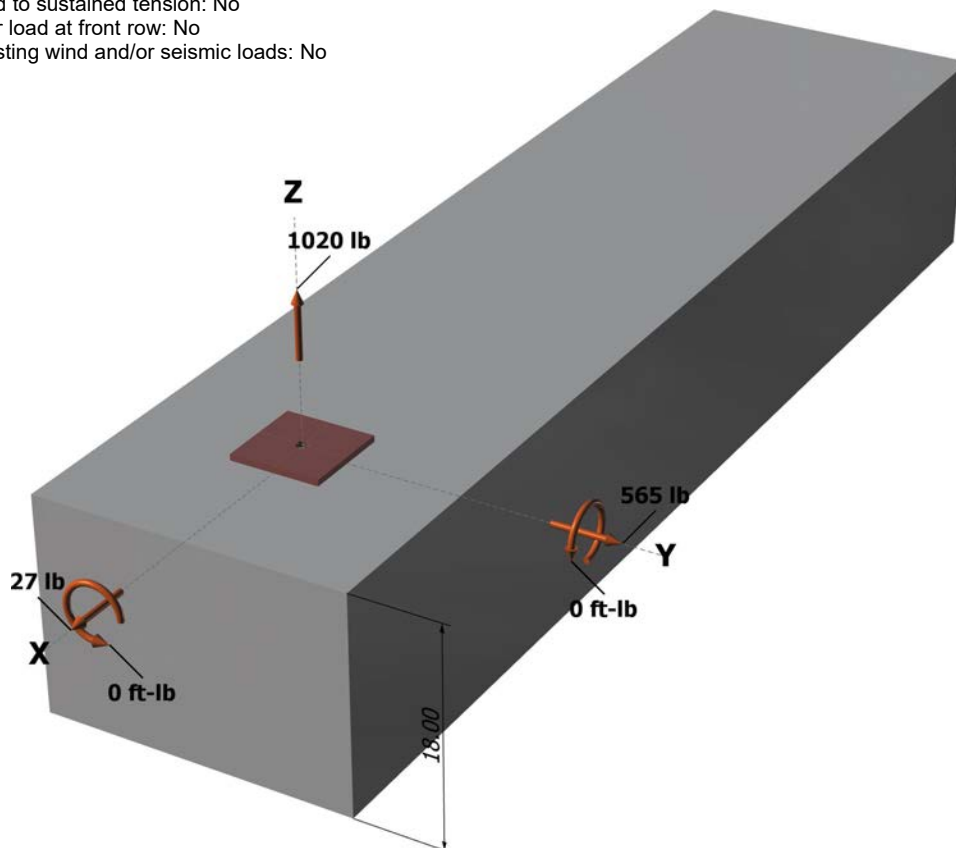
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
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Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 [www.strongtie.com](http://www.strongtie.com)



# Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
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Address:			
Phone:			
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Address:			
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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

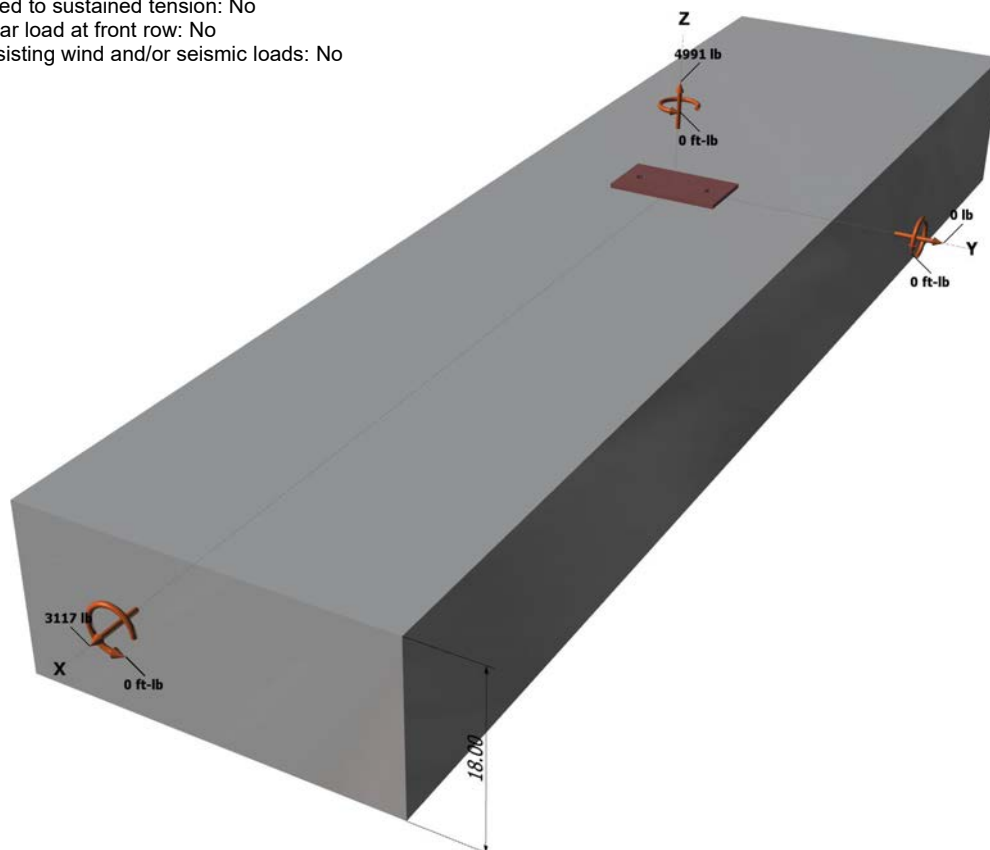
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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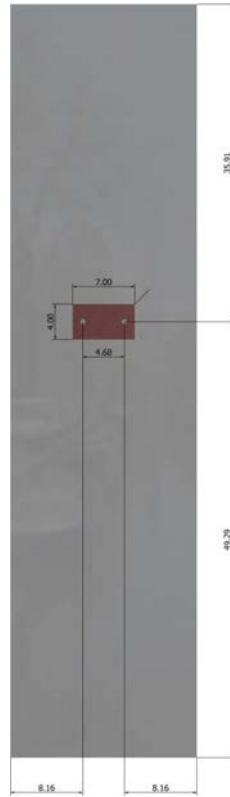




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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

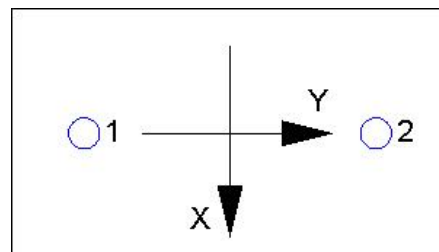
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.