



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	96 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.601 k-ft
$M_z$ =	0.234 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>78%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.344 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.775 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>99%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.247 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>12%</u>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

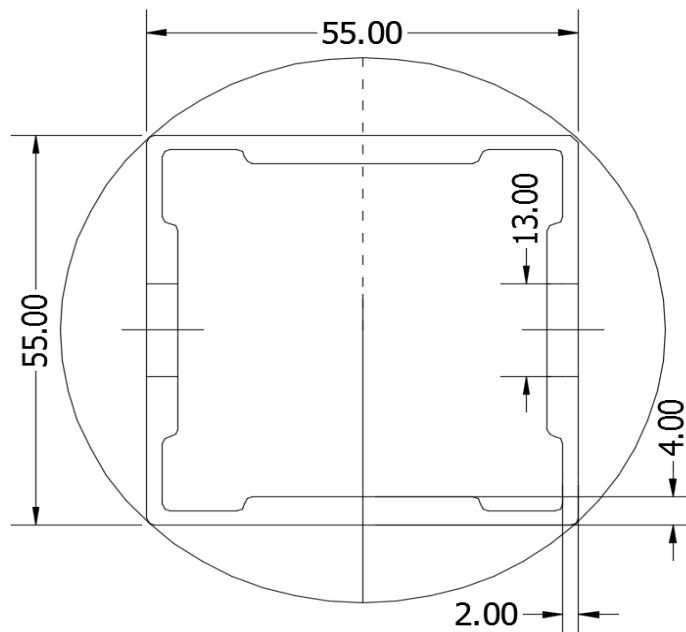
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.013 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.875 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<u>32%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.410 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<b>26%</b>



#### 5. FOUNDATION DESIGN CALCULATIONS

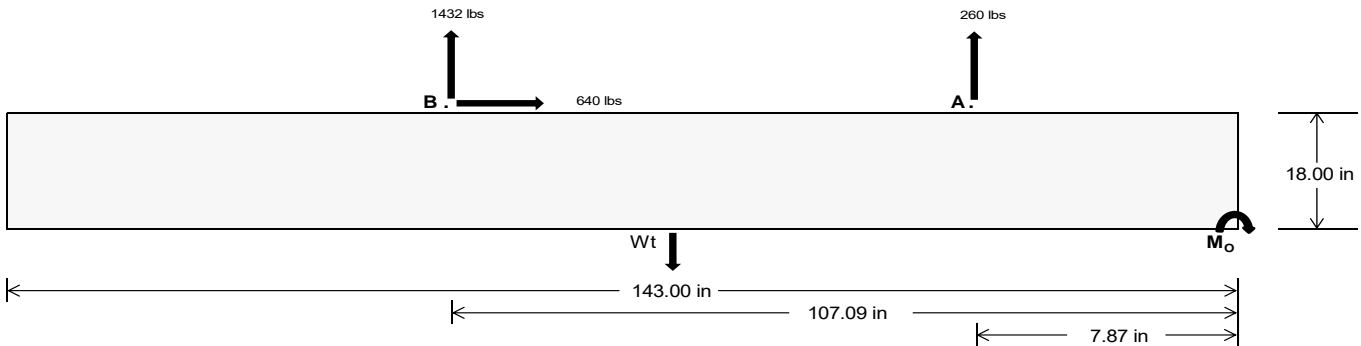
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>1091.14</b>	<b>5965.85</b>	k
Compressive Load =	<b>4220.72</b>	<b>4784.73</b>	k
Lateral Load =	<b>12.30</b>	<b>2662.66</b>	k
Moment (Weak Axis) =	<b>0.02</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 166868.8$  in-lbs  
Resisting Force Required = 2333.83 lbs  
S.F. = 1.67  
Weight Required = 3889.72 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 640.10 lbs  
Friction = 0.4  
Weight Required = 1600.24 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 640.10 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) =$  7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1404 lbs	1404 lbs	1404 lbs	1404 lbs	1600 lbs	1600 lbs	1600 lbs	1600 lbs	2137 lbs	2137 lbs	2137 lbs	2137 lbs	-519 lbs	-519 lbs	-519 lbs	-519 lbs
$F_B$	1504 lbs	1504 lbs	1504 lbs	1504 lbs	1959 lbs	1959 lbs	1959 lbs	1959 lbs	2474 lbs	2474 lbs	2474 lbs	2474 lbs	-2863 lbs	-2863 lbs	-2863 lbs	-2863 lbs
$F_V$	142 lbs	142 lbs	142 lbs	142 lbs	1136 lbs	1136 lbs	1136 lbs	1136 lbs	948 lbs	948 lbs	948 lbs	948 lbs	-1280 lbs	-1280 lbs	-1280 lbs	-1280 lbs
$P_{total}$	10468 lbs	10684 lbs	10900 lbs	11116 lbs	11118 lbs	11334 lbs	11550 lbs	11766 lbs	12171 lbs	12387 lbs	12603 lbs	12819 lbs	1153 lbs	1283 lbs	1413 lbs	1542 lbs
$M$	3199 lbs-ft	3199 lbs-ft	3199 lbs-ft	3199 lbs-ft	4378 lbs-ft	4378 lbs-ft	4378 lbs-ft	4378 lbs-ft	5417 lbs-ft	5417 lbs-ft	5417 lbs-ft	5417 lbs-ft	3818 lbs-ft	3818 lbs-ft	3818 lbs-ft	3818 lbs-ft
$e$	0.31 ft	0.30 ft	0.29 ft	0.29 ft	0.39 ft	0.39 ft	0.38 ft	0.37 ft	0.45 ft	0.44 ft	0.43 ft	0.42 ft	3.31 ft	2.98 ft	2.70 ft	2.48 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	254.8 psf	253.8 psf	252.8 psf	251.9 psf	256.5 psf	255.4 psf	254.4 psf	253.4 psf	271.7 psf	270.2 psf	268.8 psf	267.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	347.5 psf	343.9 psf	340.5 psf	337.2 psf	383.3 psf	378.7 psf	374.3 psf	370.2 psf	428.6 psf	422.8 psf	417.2 psf	412.0 psf	99.6 psf	95.6 psf	93.8 psf	93.2 psf

Maximum Bearing Pressure = 429 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

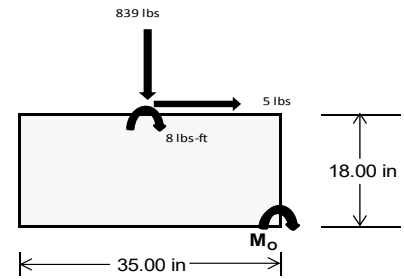
## Overturning Check

$M_o = 1208.9 \text{ ft-lbs}$   
 Resisting Force Required = 828.98 lbs  
 S.F. = 1.67  
 Weight Required = 1381.63 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	236 lbs	592 lbs	236 lbs	839 lbs	2386 lbs	839 lbs	69 lbs	173 lbs	69 lbs
$F_v$	1 lbs	0 lbs	1 lbs	5 lbs	0 lbs	5 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9595 lbs	7560 lbs	9595 lbs	9748 lbs	7560 lbs	9748 lbs	2806 lbs	7560 lbs	2806 lbs
$M$	4 lbs-ft	0 lbs-ft	4 lbs-ft	15 lbs-ft	0 lbs-ft	15 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	275.8 psf	217.5 psf	275.8 psf	279.6 psf	217.5 psf	279.6 psf	80.7 psf	217.5 psf	80.7 psf
$f_{max}$	276.3 psf	217.5 psf	276.3 psf	281.4 psf	217.5 psf	281.4 psf	80.8 psf	217.5 psf	80.8 psf



Maximum Bearing Pressure = 281 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements: 143in long x 32in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.**

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.807 k
Allowable Uplift =	1.214 k
Utilization =	<u>66%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.296 k
Allowable Uplift =	4.357 k
Utilization =	<u>53%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.247 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

#### Rear Strut

Maximum Axial Load =	4.105 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>55%</u>

#### Diagonal Strut

Maximum Axial Load =	2.041 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>28%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 96 \text{ in}$$

$$J = 0.432$$

$$265.581$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 28.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 96$$

$$J = 0.432$$

$$168.894$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.1$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max St} = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max Wk} = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} F_{cy}) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.7

$$\begin{aligned} \lambda &= 1.41345 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77788 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 13.6277 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-65.446	-65.446	0	0
2	M14	y	-65.446	-65.446	0	0
3	M15	y	-102.844	-102.844	0	0
4	M16	y	-102.844	-102.844	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	149.592	149.592	0	0
2	M14	y	114.687	114.687	0	0
3	M15	y	62.33	62.33	0	0
4	M16	y	62.33	62.33	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



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Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	58.854	1	242.969	1	.749	3	.015	2	-.004	15	.819	3
28			min	2.134	15	-334.592	3	-26.496	1	0	15	-.108	1	-.508	1
29		15	max	58.854	1	100.105	1	8.563	1	.015	2	-.004	12	1.024	3
30			min	2.134	15	-127.07	3	-.029	10	0	15	-.116	1	-.66	1
31		16	max	58.854	1	80.452	3	43.623	1	.015	2	-.002	12	1.045	3
32			min	2.134	15	-42.76	1	1.581	15	0	15	-.093	1	-.685	1
33		17	max	58.854	1	287.975	3	78.682	1	.015	2	.002	3	.881	3
34			min	2.134	15	-185.625	1	2.83	15	0	15	-.038	1	-.584	1
35		18	max	58.854	1	495.497	3	113.742	1	.015	2	.047	1	.533	3
36			min	2.134	15	-328.489	1	4.079	15	0	15	.002	15	-.355	1
37		19	max	58.854	1	703.019	3	148.801	1	.015	2	.164	1	0	1
38			min	2.134	15	-471.354	1	5.328	15	0	15	.006	15	0	3
39	M14	1	max	36.801	1	539.398	1	-5.554	15	.013	3	.198	1	0	1
40			min	1.333	15	-573.172	3	-155.124	1	-.016	1	.007	15	0	3
41		2	max	36.801	1	396.533	1	-4.305	15	.013	3	.075	1	.439	3
42			min	1.333	15	-415.292	3	-120.065	1	-.016	1	.003	15	-.416	1
43		3	max	36.801	1	253.669	1	-3.056	15	.013	3	.004	3	.738	3
44			min	1.333	15	-257.412	3	-85.005	1	-.016	1	-.016	1	-.705	1
45		4	max	36.801	1	110.804	1	-1.807	15	.013	3	0	12	.897	3
46			min	1.333	15	-99.533	3	-49.946	1	-.016	1	-.076	1	-.867	1
47		5	max	36.801	1	58.347	3	-.551	10	.013	3	-.003	12	.915	3
48			min	1.333	15	-32.061	1	-14.886	1	-.016	1	-.105	1	-.902	1
49		6	max	36.801	1	216.226	3	20.173	1	.013	3	-.004	15	.793	3
50			min	1.333	15	-174.925	1	-1.133	3	-.016	1	-.102	1	-.81	1
51		7	max	36.801	1	374.106	3	55.233	1	.013	3	-.002	15	.531	3
52			min	1.333	15	-317.79	1	.603	12	-.016	1	-.069	1	-.591	1
53		8	max	36.801	1	531.985	3	90.292	1	.013	3	.002	10	.128	3
54			min	1.333	15	-460.655	1	1.872	12	-.016	1	-.005	3	-.253	2
55		9	max	36.801	1	689.865	3	125.352	1	.013	3	.092	1	.228	1
56			min	1.333	15	-603.519	1	3.141	12	-.016	1	-.002	3	-.415	3
57		10	max	36.801	1	746.384	1	19.443	13	.013	3	.219	1	.828	1
58			min	1.333	15	-847.745	3	-160.412	1	-.016	1	.002	12	-1.098	3
59		11	max	36.801	1	603.519	1	-3.141	12	.016	1	.092	1	.228	1
60			min	1.333	15	-689.865	3	-125.352	1	-.013	3	-.002	3	-.415	3
61		12	max	36.801	1	460.655	1	-1.872	12	.016	1	.002	10	.128	3
62			min	1.333	15	-531.985	3	-90.292	1	-.013	3	-.005	3	-.253	2
63		13	max	36.801	1	317.79	1	-.603	12	.016	1	-.002	15	.531	3
64			min	1.333	15	-374.106	3	-55.233	1	-.013	3	-.069	1	-.591	1
65		14	max	36.801	1	174.925	1	1.133	3	.016	1	-.004	15	.793	3
66			min	1.333	15	-216.226	3	-20.173	1	-.013	3	-.102	1	-.81	1
67		15	max	36.801	1	32.061	1	14.886	1	.016	1	-.003	12	.915	3
68			min	1.333	15	-58.347	3	.551	10	-.013	3	-.105	1	-.902	1
69		16	max	36.801	1	99.533	3	49.946	1	.016	1	0	12	.897	3
70			min	1.333	15	-110.804	1	1.807	15	-.013	3	-.076	1	-.867	1
71		17	max	36.801	1	257.412	3	85.005	1	.016	1	.004	3	.738	3
72			min	1.333	15	-253.669	1	3.056	15	-.013	3	-.016	1	-.705	1
73		18	max	36.801	1	415.292	3	120.065	1	.016	1	.075	1	.439	3
74			min	1.333	15	-396.533	1	4.305	15	-.013	3	.003	15	-.416	1
75		19	max	36.801	1	573.172	3	155.124	1	.016	1	.198	1	0	1
76			min	1.333	15	-539.398	1	5.554	15	-.013	3	.007	15	0	3
77	M15	1	max	-1.413	15	678.818	2	-5.552	15	.016	2	.197	1	0	2
78			min	-38.831	1	-322.724	3	-155.111	1	-.011	3	.007	15	0	3
79		2	max	-1.413	15	494.63	2	-4.303	15	.016	2	.075	1	.25	3
80			min	-38.831	1	-239.308	3	-120.052	1	-.011	3	.003	15	-.522	2
81		3	max	-1.413	15	310.442	2	-3.054	15	.016	2	.004	3	.425	3
82			min	-38.831	1	-155.891	3	-84.992	1	-.011	3	-.016	1	-.879	2
83		4	max	-1.413	15	126.254	2	-1.805	15	.016	2	-.001	12	.527	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-38.831	1	-72.475	3	-49.933	1	-.011	3	-.076	1	-1.073	2
85		5	max	-1.413	15	10.941	3	-.556	15	.016	2	-.003	12	.554	3
86			min	-38.831	1	-57.934	2	-14.873	1	-.011	3	-.105	1	-1.104	2
87		6	max	-1.413	15	94.357	3	20.186	1	.016	2	-.004	15	.507	3
88			min	-38.831	1	-242.122	2	-.946	3	-.011	3	-.102	1	-.97	2
89		7	max	-1.413	15	177.774	3	55.246	1	.016	2	-.002	15	.387	3
90			min	-38.831	1	-426.31	2	.72	12	-.011	3	-.069	1	-.673	2
91		8	max	-1.413	15	261.19	3	90.305	1	.016	2	.002	10	.191	3
92			min	-38.831	1	-610.499	2	1.989	12	-.011	3	-.005	3	-.224	1
93		9	max	-1.413	15	344.606	3	125.365	1	.016	2	.092	1	.412	2
94			min	-38.831	1	-794.687	2	3.259	12	-.011	3	-.001	3	-.078	3
95		10	max	-1.413	15	978.875	2	19.451	13	.016	2	.219	1	1.2	2
96			min	-38.831	1	-266.908	12	-160.424	1	-.011	3	.003	12	-.421	3
97		11	max	-1.413	15	794.687	2	-3.259	12	.011	3	.092	1	.412	2
98			min	-38.831	1	-344.606	3	-125.365	1	-.016	2	-.001	3	-.078	3
99		12	max	-1.413	15	610.499	2	-1.989	12	.011	3	.002	10	.191	3
100			min	-38.831	1	-261.19	3	-90.305	1	-.016	2	-.005	3	-.224	1
101		13	max	-1.413	15	426.31	2	-.72	12	.011	3	-.002	15	.387	3
102			min	-38.831	1	-177.774	3	-55.246	1	-.016	2	-.069	1	-.673	2
103		14	max	-1.413	15	242.122	2	.946	3	.011	3	-.004	15	.507	3
104			min	-38.831	1	-94.357	3	-20.186	1	-.016	2	-.102	1	-.97	2
105		15	max	-1.413	15	57.934	2	14.873	1	.011	3	-.003	12	.554	3
106			min	-38.831	1	-10.941	3	.556	15	-.016	2	-.105	1	-1.104	2
107		16	max	-1.413	15	72.475	3	49.933	1	.011	3	-.001	12	.527	3
108			min	-38.831	1	-126.254	2	1.805	15	-.016	2	-.076	1	-1.073	2
109		17	max	-1.413	15	155.891	3	84.992	1	.011	3	.004	3	.425	3
110			min	-38.831	1	-310.442	2	3.054	15	-.016	2	-.016	1	-.879	2
111		18	max	-1.413	15	239.308	3	120.052	1	.011	3	.075	1	.25	3
112			min	-38.831	1	-494.63	2	4.303	15	-.016	2	.003	15	-.522	2
113		19	max	-1.413	15	322.724	3	155.111	1	.011	3	.197	1	0	2
114			min	-38.831	1	-678.818	2	5.552	15	-.016	2	.007	15	0	3
115	M16	1	max	-2.376	15	613.433	2	-5.34	15	.011	1	.166	1	0	2
116			min	-65.556	1	-271.422	3	-149.326	1	-.012	3	.006	15	0	3
117		2	max	-2.376	15	429.245	2	-4.092	15	.011	1	.049	1	.204	3
118			min	-65.556	1	-188.005	3	-114.267	1	-.012	3	.002	15	-.463	2
119		3	max	-2.376	15	245.057	2	-2.843	15	.011	1	.001	3	.334	3
120			min	-65.556	1	-104.589	3	-79.207	1	-.012	3	-.037	1	-.763	2
121		4	max	-2.376	15	60.869	2	-1.594	15	.011	1	-.002	12	.39	3
122			min	-65.556	1	-21.173	3	-44.147	1	-.012	3	-.092	1	-.899	2
123		5	max	-2.376	15	62.243	3	-.21	10	.011	1	-.004	12	.372	3
124			min	-65.556	1	-123.319	2	-9.088	1	-.012	3	-.115	1	-.871	2
125		6	max	-2.376	15	145.66	3	25.972	1	.011	1	-.004	15	.279	3
126			min	-65.556	1	-307.507	2	-.158	3	-.012	3	-.108	1	-.68	2
127		7	max	-2.376	15	229.076	3	61.031	1	.011	1	-.002	15	.113	3
128			min	-65.556	1	-491.695	2	1.226	12	-.012	3	-.069	1	-.325	2
129		8	max	-2.376	15	312.492	3	96.091	1	.011	1	.003	2	.194	2
130			min	-65.556	1	-675.883	2	2.495	12	-.012	3	-.004	3	-.128	3
131		9	max	-2.376	15	395.909	3	131.15	1	.011	1	.102	1	.877	2
132			min	-65.556	1	-860.072	2	3.765	12	-.012	3	0	3	-.443	3
133		10	max	-2.376	15	1044.26	2	20.173	13	.011	1	.234	1	1.723	2
134			min	-65.556	1	-479.325	3	-166.21	1	-.012	3	.004	12	-.832	3
135		11	max	-2.376	15	860.072	2	-3.765	12	.012	3	.102	1	.877	2
136			min	-65.556	1	-395.909	3	-131.15	1	-.011	1	0	3	-.443	3
137		12	max	-2.376	15	675.883	2	-2.495	12	.012	3	.003	2	.194	2
138			min	-65.556	1	-312.492	3	-96.091	1	-.011	1	-.004	3	-.128	3
139		13	max	-2.376	15	491.695	2	-1.226	12	.012	3	-.002	15	.113	3
140			min	-65.556	1	-229.076	3	-61.031	1	-.011	1	-.069	1	-.325	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-2.376	15	307.507	2	.158	3	.012	3	-.004	15	.279	3
142		min	-65.556	1	-145.66	3	-25.972	1	-.011	1	-.108	1	-.68	2
143	15	max	-2.376	15	123.319	2	9.088	1	.012	3	-.004	12	.372	3
144		min	-65.556	1	-62.243	3	.21	10	-.011	1	-.115	1	-.871	2
145	16	max	-2.376	15	21.173	3	44.147	1	.012	3	-.002	12	.39	3
146		min	-65.556	1	-60.869	2	1.594	15	-.011	1	-.092	1	-.899	2
147	17	max	-2.376	15	104.589	3	79.207	1	.012	3	.001	3	.334	3
148		min	-65.556	1	-245.057	2	2.843	15	-.011	1	-.037	1	-.763	2
149	18	max	-2.376	15	188.005	3	114.267	1	.012	3	.049	1	.204	3
150		min	-65.556	1	-429.245	2	4.092	15	-.011	1	.002	15	-.463	2
151	19	max	-2.376	15	271.422	3	149.326	1	.012	3	.166	1	0	2
152		min	-65.556	1	-613.433	2	5.34	15	-.011	1	.006	15	0	3
153	M2	1	max	1108.012	1	2.157	4	.693	1	0	3	0	3	1
154		min	-1303.178	3	.507	15	.025	15	0	1	0	1	0	1
155	2	max	1108.428	1	2.148	4	.693	1	0	3	0	1	0	15
156		min	-1302.866	3	.505	15	.025	15	0	1	0	15	0	4
157	3	max	1108.843	1	2.14	4	.693	1	0	3	0	1	0	15
158		min	-1302.554	3	.503	15	.025	15	0	1	0	15	-.001	4
159	4	max	1109.259	1	2.131	4	.693	1	0	3	0	1	0	15
160		min	-1302.242	3	.501	15	.025	15	0	1	0	15	-.002	4
161	5	max	1109.675	1	2.122	4	.693	1	0	3	0	1	0	15
162		min	-1301.93	3	.499	15	.025	15	0	1	0	15	-.002	4
163	6	max	1110.091	1	2.114	4	.693	1	0	3	0	1	0	15
164		min	-1301.618	3	.497	15	.025	15	0	1	0	15	-.003	4
165	7	max	1110.507	1	2.105	4	.693	1	0	3	.001	1	0	15
166		min	-1301.306	3	.495	15	.025	15	0	1	0	15	-.004	4
167	8	max	1110.923	1	2.096	4	.693	1	0	3	.001	1	0	15
168		min	-1300.994	3	.493	15	.025	15	0	1	0	15	-.004	4
169	9	max	1111.339	1	2.087	4	.693	1	0	3	.002	1	-.001	15
170		min	-1300.682	3	.491	15	.025	15	0	1	0	15	-.005	4
171	10	max	1111.755	1	2.079	4	.693	1	0	3	.002	1	-.001	15
172		min	-1300.371	3	.489	15	.025	15	0	1	0	15	-.005	4
173	11	max	1112.17	1	2.07	4	.693	1	0	3	.002	1	-.001	15
174		min	-1300.059	3	.487	15	.025	15	0	1	0	15	-.006	4
175	12	max	1112.586	1	2.061	4	.693	1	0	3	.002	1	-.002	15
176		min	-1299.747	3	.485	15	.025	15	0	1	0	15	-.007	4
177	13	max	1113.002	1	2.053	4	.693	1	0	3	.002	1	-.002	15
178		min	-1299.435	3	.483	15	.025	15	0	1	0	15	-.007	4
179	14	max	1113.418	1	2.044	4	.693	1	0	3	.003	1	-.002	15
180		min	-1299.123	3	.48	15	.025	15	0	1	0	15	-.008	4
181	15	max	1113.834	1	2.035	4	.693	1	0	3	.003	1	-.002	15
182		min	-1298.811	3	.478	15	.025	15	0	1	0	15	-.008	4
183	16	max	1114.25	1	2.026	4	.693	1	0	3	.003	1	-.002	15
184		min	-1298.499	3	.476	15	.025	15	0	1	0	15	-.009	4
185	17	max	1114.666	1	2.018	4	.693	1	0	3	.003	1	-.002	15
186		min	-1298.187	3	.474	15	.025	15	0	1	0	15	-.009	4
187	18	max	1115.082	1	2.009	4	.693	1	0	3	.003	1	-.002	15
188		min	-1297.875	3	.472	15	.025	15	0	1	0	15	-.01	4
189	19	max	1115.497	1	2	4	.693	1	0	3	.003	1	-.002	15
190		min	-1297.563	3	.47	15	.025	15	0	1	0	15	-.01	4
191	M3	1	max	538.447	2	9.101	4	.167	1	0	3	0	1	4
192		min	-676.527	3	2.139	15	.006	15	0	1	0	15	.002	15
193	2	max	538.277	2	8.227	4	.167	1	0	3	0	1	.006	4
194		min	-676.655	3	1.934	15	.006	15	0	1	0	15	.001	12
195	3	max	538.106	2	7.352	4	.167	1	0	3	0	1	.003	2
196		min	-676.782	3	1.728	15	.006	15	0	1	0	15	0	3
197	4	max	537.936	2	6.478	4	.167	1	0	3	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-676.91	3	1.523	15	.006	15	0	1	0	15	-.002	3
199	5	max	537.766	2	5.603	4	.167	1	0	3	0	1	0	15
200		min	-677.038	3	1.317	15	.006	15	0	1	0	15	-.003	3
201	6	max	537.595	2	4.729	4	.167	1	0	3	0	1	-.001	15
202		min	-677.166	3	1.112	15	.006	15	0	1	0	15	-.006	4
203	7	max	537.425	2	3.854	4	.167	1	0	3	0	1	-.002	15
204		min	-677.293	3	.906	15	.006	15	0	1	0	15	-.008	4
205	8	max	537.255	2	2.98	4	.167	1	0	3	0	1	-.002	15
206		min	-677.421	3	.7	15	.006	15	0	1	0	15	-.01	4
207	9	max	537.084	2	2.105	4	.167	1	0	3	0	1	-.003	15
208		min	-677.549	3	.495	15	.006	15	0	1	0	15	-.011	4
209	10	max	536.914	2	1.231	4	.167	1	0	3	0	1	-.003	15
210		min	-677.677	3	.289	15	.006	15	0	1	0	15	-.012	4
211	11	max	536.744	2	.439	2	.167	1	0	3	0	1	-.003	15
212		min	-677.804	3	-.015	3	.006	15	0	1	0	15	-.012	4
213	12	max	536.573	2	-.122	15	.167	1	0	3	0	1	-.003	15
214		min	-677.932	3	-.526	3	.006	15	0	1	0	15	-.012	4
215	13	max	536.403	2	-.327	15	.167	1	0	3	.001	1	-.003	15
216		min	-678.06	3	-1.392	4	.006	15	0	1	0	15	-.011	4
217	14	max	536.233	2	-.533	15	.167	1	0	3	.001	1	-.002	15
218		min	-678.188	3	-2.267	4	.006	15	0	1	0	15	-.011	4
219	15	max	536.062	2	-.738	15	.167	1	0	3	.001	1	-.002	15
220		min	-678.315	3	-3.141	4	.006	15	0	1	0	15	-.009	4
221	16	max	535.892	2	-.944	15	.167	1	0	3	.001	1	-.002	15
222		min	-678.443	3	-4.016	4	.006	15	0	1	0	15	-.008	4
223	17	max	535.722	2	-1.149	15	.167	1	0	3	.001	1	-.001	15
224		min	-678.571	3	-4.89	4	.006	15	0	1	0	15	-.005	4
225	18	max	535.551	2	-1.355	15	.167	1	0	3	.001	1	0	15
226		min	-678.699	3	-5.765	4	.006	15	0	1	0	15	-.003	4
227	19	max	535.381	2	-1.561	15	.167	1	0	3	.001	1	0	1
228		min	-678.826	3	-6.639	4	.006	15	0	1	0	15	0	1
229	M4	1	max	1164.896	1	0	1	-.353	15	0	1	0	1	0
230		min	-249.12	3	0	1	-9.801	1	0	1	0	15	0	1
231	2	max	1165.066	1	0	1	-.353	15	0	1	0	12	0	1
232		min	-248.993	3	0	1	-9.801	1	0	1	0	1	0	1
233	3	max	1165.236	1	0	1	-.353	15	0	1	0	15	0	1
234		min	-248.865	3	0	1	-9.801	1	0	1	-.001	1	0	1
235	4	max	1165.407	1	0	1	-.353	15	0	1	0	15	0	1
236		min	-248.737	3	0	1	-9.801	1	0	1	-.003	1	0	1
237	5	max	1165.577	1	0	1	-.353	15	0	1	0	15	0	1
238		min	-248.609	3	0	1	-9.801	1	0	1	-.004	1	0	1
239	6	max	1165.747	1	0	1	-.353	15	0	1	0	15	0	1
240		min	-248.482	3	0	1	-9.801	1	0	1	-.005	1	0	1
241	7	max	1165.918	1	0	1	-.353	15	0	1	0	15	0	1
242		min	-248.354	3	0	1	-9.801	1	0	1	-.006	1	0	1
243	8	max	1166.088	1	0	1	-.353	15	0	1	0	15	0	1
244		min	-248.226	3	0	1	-9.801	1	0	1	-.007	1	0	1
245	9	max	1166.258	1	0	1	-.353	15	0	1	0	15	0	1
246		min	-248.098	3	0	1	-9.801	1	0	1	-.008	1	0	1
247	10	max	1166.429	1	0	1	-.353	15	0	1	0	15	0	1
248		min	-247.971	3	0	1	-9.801	1	0	1	-.009	1	0	1
249	11	max	1166.599	1	0	1	-.353	15	0	1	0	15	0	1
250		min	-247.843	3	0	1	-9.801	1	0	1	-.01	1	0	1
251	12	max	1166.769	1	0	1	-.353	15	0	1	0	15	0	1
252		min	-247.715	3	0	1	-9.801	1	0	1	-.012	1	0	1
253	13	max	1166.94	1	0	1	-.353	15	0	1	0	15	0	1
254		min	-247.587	3	0	1	-9.801	1	0	1	-.013	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1167.11	1	0	1	-.353	15	0	1	0	15	0	1
256		min	-247.459	3	0	1	-9.801	1	0	1	-.014	1	0	1
257	15	max	1167.28	1	0	1	-.353	15	0	1	0	15	0	1
258		min	-247.332	3	0	1	-9.801	1	0	1	-.015	1	0	1
259	16	max	1167.451	1	0	1	-.353	15	0	1	0	15	0	1
260		min	-247.204	3	0	1	-9.801	1	0	1	-.016	1	0	1
261	17	max	1167.621	1	0	1	-.353	15	0	1	0	15	0	1
262		min	-247.076	3	0	1	-9.801	1	0	1	-.017	1	0	1
263	18	max	1167.791	1	0	1	-.353	15	0	1	0	15	0	1
264		min	-246.948	3	0	1	-9.801	1	0	1	-.018	1	0	1
265	19	max	1167.962	1	0	1	-.353	15	0	1	0	15	0	1
266		min	-246.821	3	0	1	-9.801	1	0	1	-.019	1	0	1
267	M6	1	max	3402.266	1	2.602	2	0	1	0	0	1	0	1
268		min	-4105.44	3	.112	3	0	1	0	1	0	1	0	1
269	2	max	3402.682	1	2.595	2	0	1	0	1	0	1	0	3
270		min	-4105.128	3	.107	3	0	1	0	1	0	1	0	2
271	3	max	3403.098	1	2.588	2	0	1	0	1	0	1	0	3
272		min	-4104.816	3	.102	3	0	1	0	1	0	1	-.001	2
273	4	max	3403.514	1	2.581	2	0	1	0	1	0	1	0	3
274		min	-4104.505	3	.097	3	0	1	0	1	0	1	-.002	2
275	5	max	3403.93	1	2.575	2	0	1	0	1	0	1	0	3
276		min	-4104.193	3	.091	3	0	1	0	1	0	1	-.003	2
277	6	max	3404.346	1	2.568	2	0	1	0	1	0	1	0	3
278		min	-4103.881	3	.086	3	0	1	0	1	0	1	-.004	2
279	7	max	3404.761	1	2.561	2	0	1	0	1	0	1	0	3
280		min	-4103.569	3	.081	3	0	1	0	1	0	1	-.004	2
281	8	max	3405.177	1	2.554	2	0	1	0	1	0	1	0	3
282		min	-4103.257	3	.076	3	0	1	0	1	0	1	-.005	2
283	9	max	3405.593	1	2.547	2	0	1	0	1	0	1	0	3
284		min	-4102.945	3	.071	3	0	1	0	1	0	1	-.006	2
285	10	max	3406.009	1	2.541	2	0	1	0	1	0	1	0	3
286		min	-4102.633	3	.066	3	0	1	0	1	0	1	-.006	2
287	11	max	3406.425	1	2.534	2	0	1	0	1	0	1	0	3
288		min	-4102.321	3	.061	3	0	1	0	1	0	1	-.007	2
289	12	max	3406.841	1	2.527	2	0	1	0	1	0	1	0	3
290		min	-4102.009	3	.056	3	0	1	0	1	0	1	-.008	2
291	13	max	3407.257	1	2.52	2	0	1	0	1	0	1	0	3
292		min	-4101.697	3	.051	3	0	1	0	1	0	1	-.009	2
293	14	max	3407.673	1	2.514	2	0	1	0	1	0	1	0	3
294		min	-4101.385	3	.046	3	0	1	0	1	0	1	-.009	2
295	15	max	3408.088	1	2.507	2	0	1	0	1	0	1	0	3
296		min	-4101.074	3	.041	3	0	1	0	1	0	1	-.01	2
297	16	max	3408.504	1	2.5	2	0	1	0	1	0	1	0	3
298		min	-4100.762	3	.035	3	0	1	0	1	0	1	-.011	2
299	17	max	3408.92	1	2.493	2	0	1	0	1	0	1	0	3
300		min	-4100.45	3	.03	3	0	1	0	1	0	1	-.011	2
301	18	max	3409.336	1	2.486	2	0	1	0	1	0	1	0	3
302		min	-4100.138	3	.025	3	0	1	0	1	0	1	-.012	2
303	19	max	3409.752	1	2.48	2	0	1	0	1	0	1	0	3
304		min	-4099.826	3	.02	3	0	1	0	1	0	1	-.013	2
305	M7	1	max	1874.908	2	9.136	4	0	1	0	0	1	.013	2
306		min	-2038.599	3	2.144	15	0	1	0	1	0	1	0	3
307	2	max	1874.737	2	8.261	4	0	1	0	1	0	1	.009	2
308		min	-2038.727	3	1.939	15	0	1	0	1	0	1	-.002	3
309	3	max	1874.567	2	7.387	4	0	1	0	1	0	1	.006	2
310		min	-2038.855	3	1.733	15	0	1	0	1	0	1	-.004	3
311	4	max	1874.397	2	6.512	4	0	1	0	1	0	1	.003	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2038.983	3	1.527	15	0	1	0	1	0	1	-.005	3
313	5	max	1874.226	2	5.638	4	0	1	0	1	0	1	0	2
314		min	-2039.11	3	1.322	15	0	1	0	1	0	1	-.007	3
315	6	max	1874.056	2	4.764	4	0	1	0	1	0	1	-.001	2
316		min	-2039.238	3	1.116	15	0	1	0	1	0	1	-.008	3
317	7	max	1873.886	2	3.889	4	0	1	0	1	0	1	-.002	15
318		min	-2039.366	3	.911	15	0	1	0	1	0	1	-.008	3
319	8	max	1873.715	2	3.015	4	0	1	0	1	0	1	-.002	15
320		min	-2039.494	3	.705	15	0	1	0	1	0	1	-.009	4
321	9	max	1873.545	2	2.186	2	0	1	0	1	0	1	-.003	15
322		min	-2039.621	3	.395	12	0	1	0	1	0	1	-.011	4
323	10	max	1873.375	2	1.505	2	0	1	0	1	0	1	-.003	15
324		min	-2039.749	3	.039	3	0	1	0	1	0	1	-.011	4
325	11	max	1873.204	2	.823	2	0	1	0	1	0	1	-.003	15
326		min	-2039.877	3	-.472	3	0	1	0	1	0	1	-.012	4
327	12	max	1873.034	2	.142	2	0	1	0	1	0	1	-.003	15
328		min	-2040.005	3	-.983	3	0	1	0	1	0	1	-.012	4
329	13	max	1872.864	2	-.323	15	0	1	0	1	0	1	-.003	15
330		min	-2040.132	3	-1.494	3	0	1	0	1	0	1	-.011	4
331	14	max	1872.693	2	-.528	15	0	1	0	1	0	1	-.002	15
332		min	-2040.26	3	-2.232	4	0	1	0	1	0	1	-.01	4
333	15	max	1872.523	2	-.734	15	0	1	0	1	0	1	-.002	15
334		min	-2040.388	3	-3.106	4	0	1	0	1	0	1	-.009	4
335	16	max	1872.353	2	-.939	15	0	1	0	1	0	1	-.002	15
336		min	-2040.516	3	-3.981	4	0	1	0	1	0	1	-.008	4
337	17	max	1872.182	2	-1.145	15	0	1	0	1	0	1	-.001	15
338		min	-2040.644	3	-4.855	4	0	1	0	1	0	1	-.005	4
339	18	max	1872.012	2	-1.35	15	0	1	0	1	0	1	0	15
340		min	-2040.771	3	-5.73	4	0	1	0	1	0	1	-.003	4
341	19	max	1871.841	2	-1.556	15	0	1	0	1	0	1	0	1
342		min	-2040.899	3	-6.604	4	0	1	0	1	0	1	0	1
343	M8	1	max	3243.64	1	0	1	0	1	0	1	0	1	1
344		min	-841.636	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3243.81	1	0	1	0	1	0	1	0	1	0	1
346		min	-841.508	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3243.981	1	0	1	0	1	0	1	0	1	0	1
348		min	-841.38	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3244.151	1	0	1	0	1	0	1	0	1	0	1
350		min	-841.253	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3244.321	1	0	1	0	1	0	1	0	1	0	1
352		min	-841.125	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3244.492	1	0	1	0	1	0	1	0	1	0	1
354		min	-840.997	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3244.662	1	0	1	0	1	0	1	0	1	0	1
356		min	-840.869	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3244.832	1	0	1	0	1	0	1	0	1	0	1
358		min	-840.742	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3245.003	1	0	1	0	1	0	1	0	1	0	1
360		min	-840.614	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3245.173	1	0	1	0	1	0	1	0	1	0	1
362		min	-840.486	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3245.343	1	0	1	0	1	0	1	0	1	0	1
364		min	-840.358	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3245.514	1	0	1	0	1	0	1	0	1	0	1
366		min	-840.23	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3245.684	1	0	1	0	1	0	1	0	1	0	1
368		min	-840.103	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3245.854	1	0	1	0	1	0	1	0	1	0	1
370			min	-839.975	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3246.025	1	0	1	0	1	0	1	0	1	0	1
372			min	-839.847	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3246.195	1	0	1	0	1	0	1	0	1	0	1
374			min	-839.719	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3246.365	1	0	1	0	1	0	1	0	1	0	1
376			min	-839.592	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3246.536	1	0	1	0	1	0	1	0	1	0	1
378			min	-839.464	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3246.706	1	0	1	0	1	0	1	0	1	0	1
380			min	-839.336	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1108.012	1	2.157	4	-.025	15	0	1	0	1	0	1
382			min	-1303.178	3	.507	15	-.693	1	0	3	0	3	0	1
383		2	max	1108.428	1	2.148	4	-.025	15	0	1	0	15	0	15
384			min	-1302.866	3	.505	15	-.693	1	0	3	0	1	0	4
385		3	max	1108.843	1	2.14	4	-.025	15	0	1	0	15	0	15
386			min	-1302.554	3	.503	15	-.693	1	0	3	0	1	-.001	4
387		4	max	1109.259	1	2.131	4	-.025	15	0	1	0	15	0	15
388			min	-1302.242	3	.501	15	-.693	1	0	3	0	1	-.002	4
389		5	max	1109.675	1	2.122	4	-.025	15	0	1	0	15	0	15
390			min	-1301.93	3	.499	15	-.693	1	0	3	0	1	-.002	4
391		6	max	1110.091	1	2.114	4	-.025	15	0	1	0	15	0	15
392			min	-1301.618	3	.497	15	-.693	1	0	3	0	1	-.003	4
393		7	max	1110.507	1	2.105	4	-.025	15	0	1	0	15	0	15
394			min	-1301.306	3	.495	15	-.693	1	0	3	-.001	1	-.004	4
395		8	max	1110.923	1	2.096	4	-.025	15	0	1	0	15	0	15
396			min	-1300.994	3	.493	15	-.693	1	0	3	-.001	1	-.004	4
397		9	max	1111.339	1	2.087	4	-.025	15	0	1	0	15	-.001	15
398			min	-1300.682	3	.491	15	-.693	1	0	3	-.002	1	-.005	4
399		10	max	1111.755	1	2.079	4	-.025	15	0	1	0	15	-.001	15
400			min	-1300.371	3	.489	15	-.693	1	0	3	-.002	1	-.005	4
401		11	max	1112.17	1	2.07	4	-.025	15	0	1	0	15	-.001	15
402			min	-1300.059	3	.487	15	-.693	1	0	3	-.002	1	-.006	4
403		12	max	1112.586	1	2.061	4	-.025	15	0	1	0	15	-.002	15
404			min	-1299.747	3	.485	15	-.693	1	0	3	-.002	1	-.007	4
405		13	max	1113.002	1	2.053	4	-.025	15	0	1	0	15	-.002	15
406			min	-1299.435	3	.483	15	-.693	1	0	3	-.002	1	-.007	4
407		14	max	1113.418	1	2.044	4	-.025	15	0	1	0	15	-.002	15
408			min	-1299.123	3	.48	15	-.693	1	0	3	-.003	1	-.008	4
409		15	max	1113.834	1	2.035	4	-.025	15	0	1	0	15	-.002	15
410			min	-1298.811	3	.478	15	-.693	1	0	3	-.003	1	-.008	4
411		16	max	1114.25	1	2.026	4	-.025	15	0	1	0	15	-.002	15
412			min	-1298.499	3	.476	15	-.693	1	0	3	-.003	1	-.009	4
413		17	max	1114.666	1	2.018	4	-.025	15	0	1	0	15	-.002	15
414			min	-1298.187	3	.474	15	-.693	1	0	3	-.003	1	-.009	4
415		18	max	1115.082	1	2.009	4	-.025	15	0	1	0	15	-.002	15
416			min	-1297.875	3	.472	15	-.693	1	0	3	-.003	1	-.01	4
417		19	max	1115.497	1	2	4	-.025	15	0	1	0	15	-.002	15
418			min	-1297.563	3	.47	15	-.693	1	0	3	-.003	1	-.01	4
419	M11	1	max	538.447	2	9.101	4	-.006	15	0	1	0	15	.01	4
420			min	-676.527	3	2.139	15	-.167	1	0	3	0	1	.002	15
421		2	max	538.277	2	8.227	4	-.006	15	0	1	0	15	.006	4
422			min	-676.655	3	1.934	15	-.167	1	0	3	0	1	.001	12
423		3	max	538.106	2	7.352	4	-.006	15	0	1	0	15	.003	2
424			min	-676.782	3	1.728	15	-.167	1	0	3	0	1	0	3
425		4	max	537.936	2	6.478	4	-.006	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-676.91	3	1.523	15	-.167	1	0	3	0	1	-.002	3
427		5	max	537.766	2	5.603	4	-.006	15	0	1	0	15	0	15
428			min	-677.038	3	1.317	15	-.167	1	0	3	0	1	-.003	3
429		6	max	537.595	2	4.729	4	-.006	15	0	1	0	15	-.001	15
430			min	-677.166	3	1.112	15	-.167	1	0	3	0	1	-.006	4
431		7	max	537.425	2	3.854	4	-.006	15	0	1	0	15	-.002	15
432			min	-677.293	3	.906	15	-.167	1	0	3	0	1	-.008	4
433		8	max	537.255	2	2.98	4	-.006	15	0	1	0	15	-.002	15
434			min	-677.421	3	.7	15	-.167	1	0	3	0	1	-.01	4
435		9	max	537.084	2	2.105	4	-.006	15	0	1	0	15	-.003	15
436			min	-677.549	3	.495	15	-.167	1	0	3	0	1	-.011	4
437		10	max	536.914	2	1.231	4	-.006	15	0	1	0	15	-.003	15
438			min	-677.677	3	.289	15	-.167	1	0	3	0	1	-.012	4
439		11	max	536.744	2	.439	2	-.006	15	0	1	0	15	-.003	15
440			min	-677.804	3	-.015	3	-.167	1	0	3	0	1	-.012	4
441		12	max	536.573	2	-.122	15	-.006	15	0	1	0	15	-.003	15
442			min	-677.932	3	-.526	3	-.167	1	0	3	0	1	-.012	4
443		13	max	536.403	2	-.327	15	-.006	15	0	1	0	15	-.003	15
444			min	-678.06	3	-1.392	4	-.167	1	0	3	-.001	1	-.011	4
445		14	max	536.233	2	-.533	15	-.006	15	0	1	0	15	-.002	15
446			min	-678.188	3	-2.267	4	-.167	1	0	3	-.001	1	-.011	4
447		15	max	536.062	2	-.738	15	-.006	15	0	1	0	15	-.002	15
448			min	-678.315	3	-3.141	4	-.167	1	0	3	-.001	1	-.009	4
449		16	max	535.892	2	-.944	15	-.006	15	0	1	0	15	-.002	15
450			min	-678.443	3	-4.016	4	-.167	1	0	3	-.001	1	-.008	4
451		17	max	535.722	2	-1.149	15	-.006	15	0	1	0	15	-.001	15
452			min	-678.571	3	-4.89	4	-.167	1	0	3	-.001	1	-.005	4
453		18	max	535.551	2	-1.355	15	-.006	15	0	1	0	15	0	15
454			min	-678.699	3	-5.765	4	-.167	1	0	3	-.001	1	-.003	4
455		19	max	535.381	2	-1.561	15	-.006	15	0	1	0	15	0	1
456			min	-678.826	3	-6.639	4	-.167	1	0	3	-.001	1	0	1
457	M12	1	max	1164.896	1	0	1	9.801	1	0	1	0	15	0	1
458			min	-249.12	3	0	1	.353	15	0	1	0	1	0	1
459		2	max	1165.066	1	0	1	9.801	1	0	1	0	1	0	1
460			min	-248.993	3	0	1	.353	15	0	1	0	12	0	1
461		3	max	1165.236	1	0	1	9.801	1	0	1	.001	1	0	1
462			min	-248.865	3	0	1	.353	15	0	1	0	15	0	1
463		4	max	1165.407	1	0	1	9.801	1	0	1	.003	1	0	1
464			min	-248.737	3	0	1	.353	15	0	1	0	15	0	1
465		5	max	1165.577	1	0	1	9.801	1	0	1	.004	1	0	1
466			min	-248.609	3	0	1	.353	15	0	1	0	15	0	1
467		6	max	1165.747	1	0	1	9.801	1	0	1	.005	1	0	1
468			min	-248.482	3	0	1	.353	15	0	1	0	15	0	1
469		7	max	1165.918	1	0	1	9.801	1	0	1	.006	1	0	1
470			min	-248.354	3	0	1	.353	15	0	1	0	15	0	1
471		8	max	1166.088	1	0	1	9.801	1	0	1	.007	1	0	1
472			min	-248.226	3	0	1	.353	15	0	1	0	15	0	1
473		9	max	1166.258	1	0	1	9.801	1	0	1	.008	1	0	1
474			min	-248.098	3	0	1	.353	15	0	1	0	15	0	1
475		10	max	1166.429	1	0	1	9.801	1	0	1	.009	1	0	1
476			min	-247.971	3	0	1	.353	15	0	1	0	15	0	1
477		11	max	1166.599	1	0	1	9.801	1	0	1	.01	1	0	1
478			min	-247.843	3	0	1	.353	15	0	1	0	15	0	1
479		12	max	1166.769	1	0	1	9.801	1	0	1	.012	1	0	1
480			min	-247.715	3	0	1	.353	15	0	1	0	15	0	1
481		13	max	1166.94	1	0	1	9.801	1	0	1	.013	1	0	1
482			min	-247.587	3	0	1	.353	15	0	1	0	15	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1167.11	1	0	1	9.801	1	0	1	.014	1	0	1
484			min	-247.459	3	0	1	.353	15	0	1	0	15	0	1
485		15	max	1167.28	1	0	1	9.801	1	0	1	.015	1	0	1
486			min	-247.332	3	0	1	.353	15	0	1	0	15	0	1
487		16	max	1167.451	1	0	1	9.801	1	0	1	.016	1	0	1
488			min	-247.204	3	0	1	.353	15	0	1	0	15	0	1
489		17	max	1167.621	1	0	1	9.801	1	0	1	.017	1	0	1
490			min	-247.076	3	0	1	.353	15	0	1	0	15	0	1
491		18	max	1167.791	1	0	1	9.801	1	0	1	.018	1	0	1
492			min	-246.948	3	0	1	.353	15	0	1	0	15	0	1
493		19	max	1167.962	1	0	1	9.801	1	0	1	.019	1	0	1
494			min	-246.821	3	0	1	.353	15	0	1	0	15	0	1
495	M1	1	max	148.806	1	702.97	3	-2.134	15	0	1	.164	1	0	15
496			min	5.328	15	-469.288	1	-58.773	1	0	3	.006	15	-.015	2
497		2	max	149.382	1	701.782	3	-2.134	15	0	1	.128	1	.278	1
498			min	5.501	15	-470.871	1	-58.773	1	0	3	.005	15	-.44	3
499		3	max	437.388	3	570.023	1	-2.109	15	0	3	.091	1	.559	1
500			min	-280.795	2	-534.974	3	-58.26	1	0	1	.003	15	-.861	3
501		4	max	437.82	3	568.44	1	-2.109	15	0	3	.055	1	.206	1
502			min	-280.219	2	-536.162	3	-58.26	1	0	1	.002	15	-.529	3
503		5	max	438.253	3	566.856	1	-2.109	15	0	3	.019	1	-.005	15
504			min	-279.643	2	-537.349	3	-58.26	1	0	1	0	15	-.196	3
505		6	max	438.685	3	565.273	1	-2.109	15	0	3	0	15	.138	3
506			min	-279.066	2	-538.537	3	-58.26	1	0	1	-.017	1	-.509	2
507		7	max	439.117	3	563.69	1	-2.109	15	0	3	-.002	15	.473	3
508			min	-278.49	2	-539.724	3	-58.26	1	0	1	-.054	1	-.848	1
509		8	max	439.549	3	562.107	1	-2.109	15	0	3	-.003	15	.808	3
510			min	-277.914	2	-540.911	3	-58.26	1	0	1	-.09	1	-1.197	1
511		9	max	450.633	3	45.104	2	-3.442	15	0	9	.058	1	.942	3
512			min	-217.545	2	.482	15	-95.104	1	0	3	.002	15	-1.362	1
513		10	max	451.065	3	43.521	2	-3.442	15	0	9	0	15	.921	3
514			min	-216.968	2	.004	15	-95.104	1	0	3	0	1	-1.377	2
515		11	max	451.497	3	41.937	2	-3.442	15	0	9	-.002	15	.9	3
516			min	-216.392	2	-1.938	4	-95.104	1	0	3	-.06	1	-1.404	2
517		12	max	462.395	3	358.921	3	-2.03	15	0	2	.088	1	.788	3
518			min	-155.951	2	-636.22	2	-56.291	1	0	3	.003	15	-1.245	2
519		13	max	462.827	3	357.734	3	-2.03	15	0	2	.053	1	.566	3
520			min	-155.375	2	-637.803	2	-56.291	1	0	3	.002	15	-.85	1
521		14	max	463.26	3	356.547	3	-2.03	15	0	2	.018	1	.344	3
522			min	-154.798	2	-639.386	2	-56.291	1	0	3	0	15	-.469	1
523		15	max	463.692	3	355.359	3	-2.03	15	0	2	0	15	.123	3
524			min	-154.222	2	-640.97	2	-56.291	1	0	3	-.017	1	-.088	1
525		16	max	464.124	3	354.172	3	-2.03	15	0	2	-.002	15	.342	2
526			min	-153.646	2	-642.553	2	-56.291	1	0	3	-.052	1	-.097	3
527		17	max	464.556	3	352.985	3	-2.03	15	0	2	-.003	15	.742	2
528			min	-153.07	2	-644.136	2	-56.291	1	0	3	-.087	1	-.316	3
529		18	max	-5.514	15	615.747	2	-2.377	15	0	3	-.005	15	.373	2
530			min	-149.898	1	-270.324	3	-65.634	1	0	2	-.126	1	-.156	3
531		19	max	-5.34	15	614.164	2	-2.377	15	0	3	-.006	15	.012	3
532			min	-149.322	1	-271.512	3	-65.634	1	0	2	-.166	1	-.011	1
533	M5	1	max	333.459	1	2329.311	3	0	1	0	1	0	1	.03	2
534			min	9.332	12	-1620.071	1	0	1	0	1	0	1	0	15
535		2	max	334.035	1	2328.124	3	0	1	0	1	0	1	1.033	1
536			min	9.62	12	-1621.655	1	0	1	0	1	0	1	-1.438	3
537		3	max	1353.395	3	1557.998	1	0	1	0	1	0	1	2.005	1
538			min	-908.06	2	-1584.215	3	0	1	0	1	0	1	-2.839	3
539		4	max	1353.827	3	1556.415	1	0	1	0	1	0	1	1.039	1





Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-907.484	2	-1585.402	3	0	1	0	1	0	1	-1.856	3
541		5	max	1354.26	3	1554.832	1	0	1	0	1	0	1	.073	1
542			min	-906.908	2	-1586.589	3	0	1	0	1	0	1	-.871	3
543		6	max	1354.692	3	1553.249	1	0	1	0	1	0	1	.114	3
544			min	-906.332	2	-1587.777	3	0	1	0	1	0	1	-.919	2
545		7	max	1355.124	3	1551.665	1	0	1	0	1	0	1	1.099	3
546			min	-905.755	2	-1588.964	3	0	1	0	1	0	1	-1.855	1
547		8	max	1355.556	3	1550.082	1	0	1	0	1	0	1	2.086	3
548			min	-905.179	2	-1590.151	3	0	1	0	1	0	1	-2.817	1
549		9	max	1368.954	3	152.402	2	0	1	0	1	0	1	2.406	3
550			min	-775.751	2	.478	15	0	1	0	1	0	1	-3.202	1
551		10	max	1369.387	3	150.819	2	0	1	0	1	0	1	2.324	3
552			min	-775.175	2	0	15	0	1	0	1	0	1	-3.252	1
553		11	max	1369.819	3	149.236	2	0	1	0	1	0	1	2.242	3
554			min	-774.598	2	-1.803	4	0	1	0	1	0	1	-3.344	2
555		12	max	1383.589	3	1019.334	3	0	1	0	1	0	1	1.962	3
556			min	-645.314	2	-1787.869	2	0	1	0	1	0	1	-2.987	2
557		13	max	1384.021	3	1018.146	3	0	1	0	1	0	1	1.33	3
558			min	-644.738	2	-1789.452	2	0	1	0	1	0	1	-1.877	2
559		14	max	1384.453	3	1016.959	3	0	1	0	1	0	1	.698	3
560			min	-644.162	2	-1791.035	2	0	1	0	1	0	1	-.812	1
561		15	max	1384.885	3	1015.771	3	0	1	0	1	0	1	.346	2
562			min	-643.585	2	-1792.618	2	0	1	0	1	0	1	0	15
563		16	max	1385.317	3	1014.584	3	0	1	0	1	0	1	1.459	2
564			min	-643.009	2	-1794.201	2	0	1	0	1	0	1	-.563	3
565		17	max	1385.749	3	1013.397	3	0	1	0	1	0	1	2.573	2
566			min	-642.433	2	-1795.784	2	0	1	0	1	0	1	-1.192	3
567		18	max	-10.355	12	2093.016	2	0	1	0	1	0	1	1.316	2
568			min	-333.003	1	-957.78	3	0	1	0	1	0	1	-.62	3
569		19	max	-10.067	12	2091.433	2	0	1	0	1	0	1	.021	1
570			min	-332.427	1	-958.967	3	0	1	0	1	0	1	-.025	3
571	M9	1	max	148.806	1	702.97	3	58.773	1	0	3	-.006	15	0	15
572			min	5.328	15	-469.288	1	2.134	15	0	1	-.164	1	-.015	2
573		2	max	149.382	1	701.782	3	58.773	1	0	3	-.005	15	.278	1
574			min	5.501	15	-470.871	1	2.134	15	0	1	-.128	1	-.44	3
575		3	max	437.388	3	570.023	1	58.26	1	0	1	-.003	15	.559	1
576			min	-280.795	2	-534.974	3	2.109	15	0	3	-.091	1	-.861	3
577		4	max	437.82	3	568.44	1	58.26	1	0	1	-.002	15	.206	1
578			min	-280.219	2	-536.162	3	2.109	15	0	3	-.055	1	-.529	3
579		5	max	438.253	3	566.856	1	58.26	1	0	1	0	15	-.005	15
580			min	-279.643	2	-537.349	3	2.109	15	0	3	-.019	1	-.196	3
581		6	max	438.685	3	565.273	1	58.26	1	0	1	.017	1	.138	3
582			min	-279.066	2	-538.537	3	2.109	15	0	3	0	15	-.509	2
583		7	max	439.117	3	563.69	1	58.26	1	0	1	.054	1	.473	3
584			min	-278.49	2	-539.724	3	2.109	15	0	3	.002	15	-.848	1
585		8	max	439.549	3	562.107	1	58.26	1	0	1	.09	1	.808	3
586			min	-277.914	2	-540.911	3	2.109	15	0	3	.003	15	-1.197	1
587		9	max	450.633	3	45.104	2	95.104	1	0	3	-.002	15	.942	3
588			min	-217.545	2	.482	15	3.442	15	0	9	-.058	1	-1.362	1
589		10	max	451.065	3	43.521	2	95.104	1	0	3	0	1	.921	3
590			min	-216.968	2	.004	15	3.442	15	0	9	0	15	-1.377	2
591		11	max	451.497	3	41.937	2	95.104	1	0	3	.06	1	.9	3
592			min	-216.392	2	-1.938	4	3.442	15	0	9	.002	15	-1.404	2
593		12	max	462.395	3	358.921	3	56.291	1	0	3	-.003	15	.788	3
594			min	-155.951	2	-636.22	2	2.03	15	0	2	-.088	1	-1.245	2
595		13	max	462.827	3	357.734	3	56.291	1	0	3	-.002	15	.566	3
596			min	-155.375	2	-637.803	2	2.03	15	0	2	-.053	1	-.85	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	463.26	3	356.547	3	56.291	1	0	3	0	15	.344	3
598		min	-154.798	2	-639.386	2	2.03	15	0	2	-.018	1	-.469	1
599	15	max	463.692	3	355.359	3	56.291	1	0	3	.017	1	.123	3
600		min	-154.222	2	-640.97	2	2.03	15	0	2	0	15	-.088	1
601	16	max	464.124	3	354.172	3	56.291	1	0	3	.052	1	.342	2
602		min	-153.646	2	-642.553	2	2.03	15	0	2	.002	15	-.097	3
603	17	max	464.556	3	352.985	3	56.291	1	0	3	.087	1	.742	2
604		min	-153.07	2	-644.136	2	2.03	15	0	2	.003	15	-.316	3
605	18	max	-5.514	15	615.747	2	65.634	1	0	2	.126	1	.373	2
606		min	-149.898	1	-270.324	3	2.377	15	0	3	.005	15	-.156	3
607	19	max	-5.34	15	614.164	2	65.634	1	0	2	.166	1	.012	3
608		min	-149.322	1	-271.512	3	2.377	15	0	3	.006	15	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.211	2	.009	3	1.43e-2	2	NC	1	NC	1
2			min	0	15	-.054	3	-.005	2	-3.449e-3	3	NC	1	NC	1
3		2	max	0	1	.13	2	.019	1	1.546e-2	2	NC	4	NC	2
4			min	0	15	.004	15	-.002	10	-3.148e-3	3	1096.985	3	9832.153	1
5		3	max	0	1	.263	3	.045	1	1.661e-2	2	NC	5	NC	2
6			min	0	15	.002	15	0	10	-2.848e-3	3	604.503	3	4233.635	1
7		4	max	0	1	.352	3	.067	1	1.777e-2	2	NC	5	NC	3
8			min	0	15	-.007	9	0	10	-2.547e-3	3	472.976	3	2878.751	1
9		5	max	0	1	.375	3	.077	1	1.892e-2	2	NC	5	NC	3
10			min	0	15	-.005	9	0	10	-2.247e-3	3	446.793	3	2502.836	1
11		6	max	0	1	.336	3	.072	1	2.008e-2	2	NC	5	NC	3
12			min	0	15	.002	15	0	10	-1.946e-3	3	491.949	3	2648.96	1
13		7	max	0	1	.246	3	.055	1	2.124e-2	2	NC	4	NC	2
14			min	0	15	.004	15	-.003	10	-1.646e-3	3	639	3	3490.619	1
15		8	max	0	1	.261	2	.029	1	2.239e-2	2	NC	4	NC	2
16			min	0	15	.006	15	-.007	10	-1.345e-3	3	1039.437	3	6572.367	1
17		9	max	0	1	.337	2	.026	3	2.355e-2	2	NC	4	NC	1
18			min	0	15	.008	15	-.013	2	-1.044e-3	3	1521.558	2	NC	1
19		10	max	0	1	.371	2	.026	3	2.471e-2	2	NC	5	NC	1
20		min	0	1	-.023	3	-.018	2	-7.439e-4	3	1201.829	2	NC	1	
21	11	max	0	15	.337	2	.026	3	2.355e-2	2	NC	4	NC	1	
22		min	0	1	.008	15	-.013	2	-1.044e-3	3	1521.558	2	NC	1	
23	12	max	0	15	.261	2	.029	1	2.239e-2	2	NC	4	NC	2	
24		min	0	1	.006	15	-.007	10	-1.345e-3	3	1039.437	3	6572.367	1	
25	13	max	0	15	.246	3	.055	1	2.124e-2	2	NC	4	NC	2	
26		min	0	1	.004	15	-.003	10	-1.646e-3	3	639	3	3490.619	1	
27	14	max	0	15	.336	3	.072	1	2.008e-2	2	NC	5	NC	3	
28		min	0	1	.002	15	0	10	-1.946e-3	3	491.949	3	2648.96	1	
29	15	max	0	15	.375	3	.077	1	1.892e-2	2	NC	5	NC	3	
30		min	0	1	-.005	9	0	10	-2.247e-3	3	446.793	3	2502.836	1	
31	16	max	0	15	.352	3	.067	1	1.777e-2	2	NC	5	NC	3	
32		min	0	1	-.007	9	0	10	-2.547e-3	3	472.976	3	2878.751	1	
33	17	max	0	15	.263	3	.045	1	1.661e-2	2	NC	5	NC	2	
34		min	0	1	.002	15	0	10	-2.848e-3	3	604.503	3	4233.635	1	
35	18	max	0	15	.13	2	.019	1	1.546e-2	2	NC	4	NC	2	
36		min	0	1	.004	15	-.002	10	-3.148e-3	3	1096.985	3	9832.153	1	
37	19	max	0	15	.211	2	.009	3	1.43e-2	2	NC	1	NC	1	
38		min	0	1	-.054	3	-.005	2	-3.449e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.39	3	.008	3	8.08e-3	1	NC	1	NC	1
40			min	0	15	-.623	2	-.004	2	-5.934e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.61	3	.012	1	9.299e-3	1	NC	5	NC	1
42		min	0	15	-.867	1	-.002	10	-6.944e-3	3	783.054	1	NC	1
43	3	max	0	1	.802	3	.034	1	1.052e-2	1	NC	5	NC	2
44		min	0	15	-1.087	1	0	10	-7.955e-3	3	412.93	1	5617.22	1
45	4	max	0	1	.949	3	.055	1	1.174e-2	1	NC	15	NC	2
46		min	0	15	-1.263	1	0	10	-8.965e-3	3	299.434	1	3522.416	1
47	5	max	0	1	1.041	3	.066	1	1.296e-2	1	NC	15	NC	3
48		min	0	15	-1.386	1	0	10	-9.976e-3	3	251.389	1	2926.394	1
49	6	max	0	1	1.076	3	.064	1	1.418e-2	1	NC	15	NC	3
50		min	0	15	-1.453	1	0	10	-1.099e-2	3	231.21	1	3006.784	1
51	7	max	0	1	1.062	3	.05	1	1.54e-2	1	9953.275	15	NC	2
52		min	0	15	-1.469	1	-.003	10	-1.2e-2	3	226.695	1	3875.526	1
53	8	max	0	1	1.015	3	.027	1	1.662e-2	1	NC	15	NC	2
54		min	0	15	-1.449	1	-.006	10	-1.301e-2	3	232.135	1	7140.159	1
55	9	max	0	1	.961	3	.023	3	1.783e-2	1	NC	15	NC	1
56		min	0	15	-1.415	1	-.011	2	-1.402e-2	3	242.254	1	NC	1
57	10	max	0	1	.934	3	.023	3	1.905e-2	1	NC	15	NC	1
58		min	0	1	-1.395	1	-.016	2	-1.503e-2	3	248.393	1	NC	1
59	11	max	0	15	.961	3	.023	3	1.783e-2	1	NC	15	NC	1
60		min	0	1	-1.415	1	-.011	2	-1.402e-2	3	242.254	1	NC	1
61	12	max	0	15	1.015	3	.027	1	1.662e-2	1	NC	15	NC	2
62		min	0	1	-1.449	1	-.006	10	-1.301e-2	3	232.135	1	7140.159	1
63	13	max	0	15	1.062	3	.05	1	1.54e-2	1	9953.275	15	NC	2
64		min	0	1	-1.469	1	-.003	10	-1.2e-2	3	226.695	1	3875.526	1
65	14	max	0	15	1.076	3	.064	1	1.418e-2	1	NC	15	NC	3
66		min	0	1	-1.453	1	0	10	-1.099e-2	3	231.21	1	3006.784	1
67	15	max	0	15	1.041	3	.066	1	1.296e-2	1	NC	15	NC	3
68		min	0	1	-1.386	1	0	10	-9.976e-3	3	251.389	1	2926.394	1
69	16	max	0	15	.949	3	.055	1	1.174e-2	1	NC	15	NC	2
70		min	0	1	-1.263	1	0	10	-8.965e-3	3	299.434	1	3522.416	1
71	17	max	0	15	.802	3	.034	1	1.052e-2	1	NC	5	NC	2
72		min	0	1	-1.087	1	0	10	-7.955e-3	3	412.93	1	5617.22	1
73	18	max	0	15	.61	3	.012	1	9.299e-3	1	NC	5	NC	1
74		min	0	1	-.867	1	-.002	10	-6.944e-3	3	783.054	1	NC	1
75	19	max	0	15	.39	3	.008	3	8.08e-3	1	NC	1	NC	1
76		min	0	1	-.623	2	-.004	2	-5.934e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.4	.007	3	4.997e-3	3	NC	1	NC	1
78		min	0	1	-.622	2	-.004	2	-8.304e-3	2	NC	1	NC	1
79	2	max	0	15	.56	3	.013	1	5.837e-3	3	NC	5	NC	1
80		min	0	1	-.901	2	-.002	10	-9.562e-3	2	688.76	2	NC	1
81	3	max	0	15	.706	3	.035	1	6.676e-3	3	NC	5	NC	2
82		min	0	1	-1.148	2	0	10	-1.082e-2	2	365.242	2	5581.144	1
83	4	max	0	15	.825	3	.055	1	7.515e-3	3	NC	15	NC	3
84		min	0	1	-1.34	2	0	10	-1.208e-2	2	267.291	2	3502.905	1
85	5	max	0	15	.911	3	.066	1	8.354e-3	3	NC	15	NC	3
86		min	0	1	-1.466	2	0	10	-1.334e-2	2	227.351	2	2910	1
87	6	max	0	15	.964	3	.065	1	9.193e-3	3	NC	15	NC	3
88		min	0	1	-1.524	2	0	10	-1.459e-2	2	212.779	2	2987.116	1
89	7	max	0	15	.984	3	.05	1	1.003e-2	3	9976.485	15	NC	2
90		min	0	1	-1.522	2	-.003	10	-1.585e-2	2	213.287	2	3840.067	1
91	8	max	0	15	.981	3	.028	1	1.087e-2	3	NC	15	NC	2
92		min	0	1	-1.479	2	-.006	10	-1.711e-2	2	224.121	2	7014.388	1
93	9	max	0	15	.966	3	.021	3	1.171e-2	3	NC	15	NC	1
94		min	0	1	-1.423	2	-.01	2	-1.837e-2	2	239.793	2	NC	1
95	10	max	0	1	.957	3	.021	3	1.255e-2	3	NC	15	NC	1
96		min	0	1	-1.394	1	-.015	2	-1.963e-2	2	248.646	1	NC	1
97	11	max	0	1	.966	3	.021	3	1.171e-2	3	NC	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-1.423	2	-.01	2	-1.837e-2	2	239.793	2	NC	1
99		12	max	0	1	.981	3	.028	1	1.087e-2	3	NC	15	NC	2
100			min	0	15	-1.479	2	-.006	10	-1.711e-2	2	224.121	2	7014.388	1
101		13	max	0	1	.984	3	.05	1	1.003e-2	3	9976.485	15	NC	2
102			min	0	15	-1.522	2	-.003	10	-1.585e-2	2	213.287	2	3840.067	1
103		14	max	0	1	.964	3	.065	1	9.193e-3	3	NC	15	NC	3
104			min	0	15	-1.524	2	0	10	-1.459e-2	2	212.779	2	2987.116	1
105		15	max	0	1	.911	3	.066	1	8.354e-3	3	NC	15	NC	3
106			min	0	15	-1.466	2	0	10	-1.334e-2	2	227.351	2	2910	1
107		16	max	0	1	.825	3	.055	1	7.515e-3	3	NC	15	NC	3
108			min	0	15	-1.34	2	0	10	-1.208e-2	2	267.291	2	3502.905	1
109		17	max	0	1	.706	3	.035	1	6.676e-3	3	NC	5	NC	2
110			min	0	15	-1.148	2	0	10	-1.082e-2	2	365.242	2	5581.144	1
111		18	max	0	1	.56	3	.013	1	5.837e-3	3	NC	5	NC	1
112			min	0	15	-.901	2	-.002	10	-9.562e-3	2	688.76	2	NC	1
113		19	max	0	1	.4	3	.007	3	4.997e-3	3	NC	1	NC	1
114			min	0	15	-.622	2	-.004	2	-8.304e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.197	1	.006	3	9.43e-3	3	NC	1	NC	1
116			min	0	1	-.141	3	-.004	2	-1.282e-2	1	NC	1	NC	1
117		2	max	0	15	.08	1	.019	1	1.04e-2	3	NC	4	NC	2
118			min	0	1	-.096	3	-.001	10	-1.365e-2	1	1458.58	2	9917.294	1
119		3	max	0	15	.013	9	.045	1	1.137e-2	3	NC	5	NC	2
120			min	0	1	-.063	3	0	10	-1.449e-2	1	816.779	2	4242.55	1
121		4	max	0	15	.004	13	.067	1	1.234e-2	3	NC	5	NC	3
122			min	0	1	-.103	2	.002	10	-1.532e-2	1	658.631	2	2871.936	1
123		5	max	0	15	.005	4	.077	1	1.331e-2	3	NC	5	NC	3
124			min	0	1	-.104	2	.002	10	-1.615e-2	1	656.541	2	2485.525	1
125		6	max	0	15	.021	9	.074	1	1.429e-2	3	NC	5	NC	3
126			min	0	1	-.097	3	.001	10	-1.699e-2	1	801.355	2	2613.839	1
127		7	max	0	15	.086	1	.057	1	1.526e-2	3	NC	3	NC	2
128			min	0	1	-.149	3	-.001	10	-1.782e-2	1	1324.349	2	3402.866	1
129		8	max	0	15	.193	1	.031	1	1.623e-2	3	NC	1	NC	2
130			min	0	1	-.208	3	-.004	10	-1.865e-2	1	2875.288	3	6189.095	1
131		9	max	0	15	.288	1	.019	3	1.72e-2	3	NC	4	NC	1
132			min	0	1	-.258	3	-.009	2	-1.949e-2	1	1641.557	3	NC	1
133		10	max	0	1	.33	1	.018	3	1.817e-2	3	NC	5	NC	1
134			min	0	1	-.28	3	-.014	2	-2.032e-2	1	1381.211	3	NC	1
135		11	max	0	1	.288	1	.019	3	1.72e-2	3	NC	4	NC	1
136			min	0	15	-.258	3	-.009	2	-1.949e-2	1	1641.557	3	NC	1
137		12	max	0	1	.193	1	.031	1	1.623e-2	3	NC	1	NC	2
138			min	0	15	-.208	3	-.004	10	-1.865e-2	1	2875.288	3	6189.095	1
139		13	max	0	1	.086	1	.057	1	1.526e-2	3	NC	3	NC	2
140			min	0	15	-.149	3	-.001	10	-1.782e-2	1	1324.349	2	3402.866	1
141		14	max	0	1	.021	9	.074	1	1.429e-2	3	NC	5	NC	3
142			min	0	15	-.097	3	.001	10	-1.699e-2	1	801.355	2	2613.839	1
143		15	max	0	1	.005	4	.077	1	1.331e-2	3	NC	5	NC	3
144			min	0	15	-.104	2	.002	10	-1.615e-2	1	656.541	2	2485.525	1
145		16	max	0	1	.004	13	.067	1	1.234e-2	3	NC	5	NC	3
146			min	0	15	-.103	2	.002	10	-1.532e-2	1	658.631	2	2871.936	1
147		17	max	0	1	.013	9	.045	1	1.137e-2	3	NC	5	NC	2
148			min	0	15	-.063	3	0	10	-1.449e-2	1	816.779	2	4242.55	1
149		18	max	0	1	.08	1	.019	1	1.04e-2	3	NC	4	NC	2
150			min	0	15	-.096	3	-.001	10	-1.365e-2	1	1458.58	2	9917.294	1
151		19	max	0	1	.197	1	.006	3	9.43e-3	3	NC	1	NC	1
152			min	0	15	-.141	3	-.004	2	-1.282e-2	1	NC	1	NC	1
153	M2	1	max	.006	1	.007	2	.008	1	-6.096e-6	15	NC	1	NC	2
154			min	-.008	3	-.012	3	0	15	-1.683e-4	1	8314.684	2	7932.595	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	1	.006	2	.007	1	-5.712e-6	15	NC	1	NC	2
156		min	-.007	3	-.011	3	0	15	-1.577e-4	1	9618.018	2	8649.14	1
157	3	max	.006	1	.005	2	.006	1	-5.328e-6	15	NC	1	NC	2
158		min	-.007	3	-.011	3	0	15	-1.471e-4	1	NC	1	9502.462	1
159	4	max	.005	1	.004	2	.006	1	-4.945e-6	15	NC	1	NC	1
160		min	-.006	3	-.011	3	0	15	-1.365e-4	1	NC	1	NC	1
161	5	max	.005	1	.003	2	.005	1	-4.561e-6	15	NC	1	NC	1
162		min	-.006	3	-.01	3	0	15	-1.259e-4	1	NC	1	NC	1
163	6	max	.005	1	.003	2	.005	1	-4.177e-6	15	NC	1	NC	1
164		min	-.005	3	-.01	3	0	15	-1.152e-4	1	NC	1	NC	1
165	7	max	.004	1	.002	2	.004	1	-3.793e-6	15	NC	1	NC	1
166		min	-.005	3	-.009	3	0	15	-1.046e-4	1	NC	1	NC	1
167	8	max	.004	1	0	2	.003	1	-3.409e-6	15	NC	1	NC	1
168		min	-.005	3	-.009	3	0	15	-9.399e-5	1	NC	1	NC	1
169	9	max	.004	1	0	2	.003	1	-3.026e-6	15	NC	1	NC	1
170		min	-.004	3	-.008	3	0	15	-8.337e-5	1	NC	1	NC	1
171	10	max	.003	1	0	2	.002	1	-2.642e-6	15	NC	1	NC	1
172		min	-.004	3	-.008	3	0	15	-7.275e-5	1	NC	1	NC	1
173	11	max	.003	1	0	2	.002	1	-2.258e-6	15	NC	1	NC	1
174		min	-.003	3	-.007	3	0	15	-6.213e-5	1	NC	1	NC	1
175	12	max	.003	1	-.001	15	.002	1	-1.874e-6	15	NC	1	NC	1
176		min	-.003	3	-.006	3	0	15	-5.15e-5	1	NC	1	NC	1
177	13	max	.002	1	-.001	15	.001	1	-1.491e-6	15	NC	1	NC	1
178		min	-.003	3	-.006	3	0	15	-4.088e-5	1	NC	1	NC	1
179	14	max	.002	1	0	15	0	1	-1.107e-6	15	NC	1	NC	1
180		min	-.002	3	-.005	3	0	15	-3.026e-5	1	NC	1	NC	1
181	15	max	.001	1	0	15	0	1	-7.23e-7	15	NC	1	NC	1
182		min	-.002	3	-.004	3	0	15	-1.964e-5	1	NC	1	NC	1
183	16	max	.001	1	0	15	0	1	-3.392e-7	15	NC	1	NC	1
184		min	-.001	3	-.003	3	0	15	-9.018e-6	1	NC	1	NC	1
185	17	max	0	1	0	15	0	1	1.603e-6	1	NC	1	NC	1
186		min	0	3	-.002	4	0	15	-7.129e-7	3	NC	1	NC	1
187	18	max	0	1	0	15	0	1	1.222e-5	1	NC	1	NC	1
188		min	0	3	-.001	4	0	15	2.936e-7	12	NC	1	NC	1
189	19	max	0	1	0	1	0	1	2.285e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	8.121e-7	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	-2.514e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-7.045e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	1.491e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	5.384e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	3.687e-5	1	NC	1	NC	1
196		min	0	2	-.005	4	0	15	1.328e-6	15	NC	1	NC	1
197	4	max	.001	3	-.002	15	0	1	5.883e-5	1	NC	1	NC	1
198		min	0	2	-.008	4	0	15	2.118e-6	15	NC	1	NC	1
199	5	max	.001	3	-.003	15	0	1	8.079e-5	1	NC	1	NC	1
200		min	-.001	2	-.011	4	0	15	2.908e-6	15	9256.277	4	NC	1
201	6	max	.002	3	-.003	15	0	1	1.028e-4	1	NC	1	NC	1
202		min	-.001	2	-.014	4	0	15	3.697e-6	15	7430.527	4	NC	1
203	7	max	.002	3	-.004	15	0	1	1.247e-4	1	NC	5	NC	1
204		min	-.002	2	-.016	4	0	15	4.487e-6	15	6334.963	4	NC	1
205	8	max	.003	3	-.004	15	.001	1	1.467e-4	1	NC	5	NC	1
206		min	-.002	2	-.018	4	0	15	5.277e-6	15	5658.669	4	NC	1
207	9	max	.003	3	-.005	15	.001	1	1.686e-4	1	NC	5	NC	1
208		min	-.002	2	-.02	4	0	15	6.067e-6	15	5255.632	4	NC	1
209	10	max	.003	3	-.005	15	.002	1	1.906e-4	1	NC	5	NC	1
210		min	-.003	2	-.021	4	0	15	6.857e-6	15	5054.679	4	NC	1
211	11	max	.004	3	-.005	15	.002	1	2.125e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.003	2	-.021	4	0	15	7.646e-6	15	5025.851	4	NC	1
213		max	.004	3	-.005	15	.003	1	2.345e-4	1	NC	5	NC	1
214		min	-.003	2	-.02	4	0	15	8.436e-6	15	5168.868	4	NC	1
215		max	.004	3	-.004	15	.003	1	2.565e-4	1	NC	5	NC	1
216		min	-.004	2	-.019	4	0	15	9.226e-6	15	5514.444	4	NC	1
217		max	.005	3	-.004	15	.004	1	2.784e-4	1	NC	5	NC	1
218		min	-.004	2	-.017	4	0	15	1.002e-5	15	6140.532	4	NC	1
219		max	.005	3	-.003	15	.004	1	3.004e-4	1	NC	2	NC	1
220		min	-.004	2	-.015	4	0	15	1.081e-5	15	7221.157	4	NC	1
221		max	.006	3	-.003	15	.005	1	3.223e-4	1	NC	1	NC	1
222		min	-.004	2	-.012	4	0	15	1.16e-5	15	9178.741	4	NC	1
223		max	.006	3	-.002	15	.005	1	3.443e-4	1	NC	1	NC	1
224		min	-.005	2	-.008	4	0	15	1.239e-5	15	NC	1	NC	1
225		max	.006	3	-.001	15	.006	1	3.663e-4	1	NC	1	NC	1
226		min	-.005	2	-.005	1	0	15	1.317e-5	15	NC	1	NC	1
227		max	.007	3	0	15	.007	1	3.882e-4	1	NC	1	NC	1
228		min	-.005	2	-.002	1	0	15	1.396e-5	15	NC	1	NC	1
229	M4	max	.003	1	.005	2	0	15	6.258e-5	1	NC	1	NC	3
230		min	0	3	-.007	3	-.007	1	2.273e-6	15	NC	1	3460.072	1
231		max	.003	1	.005	2	0	15	6.258e-5	1	NC	1	NC	2
232		min	0	3	-.006	3	-.007	1	2.273e-6	15	NC	1	3763.117	1
233		max	.002	1	.004	2	0	15	6.258e-5	1	NC	1	NC	2
234		min	0	3	-.006	3	-.006	1	2.273e-6	15	NC	1	4123.756	1
235		max	.002	1	.004	2	0	15	6.258e-5	1	NC	1	NC	2
236		min	0	3	-.006	3	-.005	1	2.273e-6	15	NC	1	4556.956	1
237		max	.002	1	.004	2	0	15	6.258e-5	1	NC	1	NC	2
238		min	0	3	-.005	3	-.005	1	2.273e-6	15	NC	1	5083.057	1
239		max	.002	1	.003	2	0	15	6.258e-5	1	NC	1	NC	2
240		min	0	3	-.005	3	-.004	1	2.273e-6	15	NC	1	5730.267	1
241		max	.002	1	.003	2	0	15	6.258e-5	1	NC	1	NC	2
242		min	0	3	-.004	3	-.004	1	2.273e-6	15	NC	1	6538.624	1
243		max	.002	1	.003	2	0	15	6.258e-5	1	NC	1	NC	2
244		min	0	3	-.004	3	-.003	1	2.273e-6	15	NC	1	7566.526	1
245		max	.002	1	.003	2	0	15	6.258e-5	1	NC	1	NC	2
246		min	0	3	-.004	3	-.003	1	2.273e-6	15	NC	1	8901.886	1
247		max	.001	1	.002	2	0	15	6.258e-5	1	NC	1	NC	1
248		min	0	3	-.003	3	-.002	1	2.273e-6	15	NC	1	NC	1
249		max	.001	1	.002	2	0	15	6.258e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.002	1	2.273e-6	15	NC	1	NC	1
251		max	.001	1	.002	2	0	15	6.258e-5	1	NC	1	NC	1
252		min	0	3	-.003	3	-.001	1	2.273e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	6.258e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	-.001	1	2.273e-6	15	NC	1	NC	1
255		max	0	1	.001	2	0	15	6.258e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	2.273e-6	15	NC	1	NC	1
257		max	0	1	.001	2	0	15	6.258e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	2.273e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	6.258e-5	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	2.273e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.258e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	2.273e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.258e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.273e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.258e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.273e-6	15	NC	1	NC	1
267	M6	max	.02	1	.025	2	0	1	0	1	NC	3	NC	1
268		min	-.024	3	-.036	3	0	1	0	1	2384.47	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.019	1	.023	2	0	1	0	1	NC	3	NC	1
270		min	-.023	3	-.034	3	0	1	0	1	2609.481	2	NC	1
271	3	max	.018	1	.021	2	0	1	0	1	NC	3	NC	1
272		min	-.021	3	-.032	3	0	1	0	1	2879.38	2	NC	1
273	4	max	.017	1	.019	2	0	1	0	1	NC	3	NC	1
274		min	-.02	3	-.03	3	0	1	0	1	3206.586	2	NC	1
275	5	max	.015	1	.017	2	0	1	0	1	NC	3	NC	1
276		min	-.019	3	-.028	3	0	1	0	1	3608.26	2	NC	1
277	6	max	.014	1	.015	2	0	1	0	1	NC	3	NC	1
278		min	-.017	3	-.026	3	0	1	0	1	4108.67	2	NC	1
279	7	max	.013	1	.013	2	0	1	0	1	NC	3	NC	1
280		min	-.016	3	-.024	3	0	1	0	1	4743.053	2	NC	1
281	8	max	.012	1	.011	2	0	1	0	1	NC	1	NC	1
282		min	-.015	3	-.022	3	0	1	0	1	5564.181	2	NC	1
283	9	max	.011	1	.009	2	0	1	0	1	NC	1	NC	1
284		min	-.013	3	-.02	3	0	1	0	1	6654.054	2	NC	1
285	10	max	.01	1	.007	2	0	1	0	1	NC	1	NC	1
286		min	-.012	3	-.018	3	0	1	0	1	8145.853	2	NC	1
287	11	max	.009	1	.006	2	0	1	0	1	NC	1	NC	1
288		min	-.011	3	-.016	3	0	1	0	1	NC	1	NC	1
289	12	max	.008	1	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.009	3	-.014	3	0	1	0	1	NC	1	NC	1
291	13	max	.007	1	.003	2	0	1	0	1	NC	1	NC	1
292		min	-.008	3	-.012	3	0	1	0	1	NC	1	NC	1
293	14	max	.006	1	.002	2	0	1	0	1	NC	1	NC	1
294		min	-.007	3	-.01	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	.001	2	0	1	0	1	NC	1	NC	1
296		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.004	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	15	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.01	3	0	1	0	1	NC	1	NC	1
313	5	max	.004	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.013	3	0	1	0	1	8548.098	3	NC	1
315	6	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.015	3	0	1	0	1	7183.938	3	NC	1
317	7	max	.007	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.006	2	-.017	3	0	1	0	1	6363.011	3	NC	1
319	8	max	.008	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.007	2	-.019	3	0	1	0	1	5770.698	4	NC	1
321	9	max	.009	3	-.005	15	0	1	0	1	NC	2	NC	1
322		min	-.008	2	-.02	4	0	1	0	1	5352.443	4	NC	1
323	10	max	.01	3	-.005	15	0	1	0	1	NC	5	NC	1
324		min	-.009	2	-.021	4	0	1	0	1	5142.011	4	NC	1
325	11	max	.011	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.01	2	-.021	4	0	1	0	1	5107.889	4	NC	1
327		12	max	.012	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.011	2	-.02	4	0	1	0	1	5249.112	4	NC	1
329		13	max	.013	3	-.004	15	0	1	0	1	NC	2	NC	1
330			min	-.012	2	-.019	4	0	1	0	1	5596.376	4	NC	1
331		14	max	.014	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.013	2	-.017	4	0	1	0	1	6228.378	4	NC	1
333		15	max	.016	3	-.003	15	0	1	0	1	NC	1	NC	1
334			min	-.014	2	-.015	4	0	1	0	1	7321.226	4	NC	1
335		16	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.015	2	-.012	4	0	1	0	1	9302.713	4	NC	1
337		17	max	.018	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.016	2	-.01	1	0	1	0	1	NC	1	NC	1
339		18	max	.019	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.017	2	-.008	1	0	1	0	1	NC	1	NC	1
341		19	max	.02	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.018	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.017	2	0	1	0	1	NC	1	NC	1
344			min	-.002	3	-.02	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.016	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.019	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	1	.015	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.018	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.014	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.017	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
352			min	-.002	3	-.016	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
354			min	-.001	3	-.015	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.011	2	0	1	0	1	NC	1	NC	1
356			min	-.001	3	-.013	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
358			min	-.001	3	-.012	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.01	2	0	1	0	1	NC	1	NC	1
360			min	-.001	3	-.011	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.009	2	0	1	0	1	NC	1	NC	1
362			min	-.001	3	-.01	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.008	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.007	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	1	.007	2	0	15	1.683e-4	1	NC	1	NC	2
382			min	-.008	3	-.012	3	-.008	1	6.096e-6	15	8314.684	2	7932.595	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.006	1	.006	2	0	15	1.577e-4	1	NC	1	NC	2
384			min	-.007	3	-.011	3	-.007	1	5.712e-6	15	9618.018	2	8649.14	1
385		3	max	.006	1	.005	2	0	15	1.471e-4	1	NC	1	NC	2
386			min	-.007	3	-.011	3	-.006	1	5.328e-6	15	NC	1	9502.462	1
387		4	max	.005	1	.004	2	0	15	1.365e-4	1	NC	1	NC	1
388			min	-.006	3	-.011	3	-.006	1	4.945e-6	15	NC	1	NC	1
389		5	max	.005	1	.003	2	0	15	1.259e-4	1	NC	1	NC	1
390			min	-.006	3	-.01	3	-.005	1	4.561e-6	15	NC	1	NC	1
391		6	max	.005	1	.003	2	0	15	1.152e-4	1	NC	1	NC	1
392			min	-.005	3	-.01	3	-.005	1	4.177e-6	15	NC	1	NC	1
393		7	max	.004	1	.002	2	0	15	1.046e-4	1	NC	1	NC	1
394			min	-.005	3	-.009	3	-.004	1	3.793e-6	15	NC	1	NC	1
395		8	max	.004	1	0	2	0	15	9.399e-5	1	NC	1	NC	1
396			min	-.005	3	-.009	3	-.003	1	3.409e-6	15	NC	1	NC	1
397		9	max	.004	1	0	2	0	15	8.337e-5	1	NC	1	NC	1
398			min	-.004	3	-.008	3	-.003	1	3.026e-6	15	NC	1	NC	1
399		10	max	.003	1	0	2	0	15	7.275e-5	1	NC	1	NC	1
400			min	-.004	3	-.008	3	-.002	1	2.642e-6	15	NC	1	NC	1
401		11	max	.003	1	0	2	0	15	6.213e-5	1	NC	1	NC	1
402			min	-.003	3	-.007	3	-.002	1	2.258e-6	15	NC	1	NC	1
403		12	max	.003	1	-.001	15	0	15	5.15e-5	1	NC	1	NC	1
404			min	-.003	3	-.006	3	-.002	1	1.874e-6	15	NC	1	NC	1
405		13	max	.002	1	-.001	15	0	15	4.088e-5	1	NC	1	NC	1
406			min	-.003	3	-.006	3	-.001	1	1.491e-6	15	NC	1	NC	1
407		14	max	.002	1	0	15	0	15	3.026e-5	1	NC	1	NC	1
408			min	-.002	3	-.005	3	0	1	1.107e-6	15	NC	1	NC	1
409		15	max	.001	1	0	15	0	15	1.964e-5	1	NC	1	NC	1
410			min	-.002	3	-.004	3	0	1	7.23e-7	15	NC	1	NC	1
411		16	max	.001	1	0	15	0	15	9.018e-6	1	NC	1	NC	1
412			min	-.001	3	-.003	3	0	1	3.392e-7	15	NC	1	NC	1
413		17	max	0	1	0	15	0	15	7.129e-7	3	NC	1	NC	1
414			min	0	3	-.002	4	0	1	-1.603e-6	1	NC	1	NC	1
415		18	max	0	1	0	15	0	15	-2.936e-7	12	NC	1	NC	1
416			min	0	3	-.001	4	0	1	-1.222e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	-8.121e-7	15	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.285e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	7.045e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	2.514e-7	15	NC	1	NC	1
421		2	max	0	3	0	15	0	15	-5.384e-7	15	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.491e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	0	15	-1.328e-6	15	NC	1	NC	1
424			min	0	2	-.005	4	0	1	-3.687e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	0	15	-2.118e-6	15	NC	1	NC	1
426			min	0	2	-.008	4	0	1	-5.883e-5	1	NC	1	NC	1
427		5	max	.001	3	-.003	15	0	15	-2.908e-6	15	NC	1	NC	1
428			min	-.001	2	-.011	4	0	1	-8.079e-5	1	9256.277	4	NC	1
429		6	max	.002	3	-.003	15	0	15	-3.697e-6	15	NC	1	NC	1
430			min	-.001	2	-.014	4	0	1	-1.028e-4	1	7430.527	4	NC	1
431		7	max	.002	3	-.004	15	0	15	-4.487e-6	15	NC	5	NC	1
432			min	-.002	2	-.016	4	0	1	-1.247e-4	1	6334.963	4	NC	1
433		8	max	.003	3	-.004	15	0	15	-5.277e-6	15	NC	5	NC	1
434			min	-.002	2	-.018	4	-.001	1	-1.467e-4	1	5658.669	4	NC	1
435		9	max	.003	3	-.005	15	0	15	-6.067e-6	15	NC	5	NC	1
436			min	-.002	2	-.02	4	-.001	1	-1.686e-4	1	5255.632	4	NC	1
437		10	max	.003	3	-.005	15	0	15	-6.857e-6	15	NC	5	NC	1
438			min	-.003	2	-.021	4	-.002	1	-1.906e-4	1	5054.679	4	NC	1
439		11	max	.004	3	-.005	15	0	15	-7.646e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.003	2	-.021	4	-.002	1	-2.125e-4	1	5025.851	4	NC	1
441		max	.004	3	-.005	15	0	15	-8.436e-6	15	NC	5	NC	1
442		min	-.003	2	-.02	4	-.003	1	-2.345e-4	1	5168.868	4	NC	1
443		max	.004	3	-.004	15	0	15	-9.226e-6	15	NC	5	NC	1
444		min	-.004	2	-.019	4	-.003	1	-2.565e-4	1	5514.444	4	NC	1
445		max	.005	3	-.004	15	0	15	-1.002e-5	15	NC	5	NC	1
446		min	-.004	2	-.017	4	-.004	1	-2.784e-4	1	6140.532	4	NC	1
447		max	.005	3	-.003	15	0	15	-1.081e-5	15	NC	2	NC	1
448		min	-.004	2	-.015	4	-.004	1	-3.004e-4	1	7221.157	4	NC	1
449		max	.006	3	-.003	15	0	15	-1.16e-5	15	NC	1	NC	1
450		min	-.004	2	-.012	4	-.005	1	-3.223e-4	1	9178.741	4	NC	1
451		max	.006	3	-.002	15	0	15	-1.239e-5	15	NC	1	NC	1
452		min	-.005	2	-.008	4	-.005	1	-3.443e-4	1	NC	1	NC	1
453		max	.006	3	-.001	15	0	15	-1.317e-5	15	NC	1	NC	1
454		min	-.005	2	-.005	1	-.006	1	-3.663e-4	1	NC	1	NC	1
455		max	.007	3	0	15	0	15	-1.396e-5	15	NC	1	NC	1
456		min	-.005	2	-.002	1	-.007	1	-3.882e-4	1	NC	1	NC	1
457	M12	max	.003	1	.005	2	.007	1	-2.273e-6	15	NC	1	NC	3
458		min	0	3	-.007	3	0	15	-6.258e-5	1	NC	1	3460.072	1
459		max	.003	1	.005	2	.007	1	-2.273e-6	15	NC	1	NC	2
460		min	0	3	-.006	3	0	15	-6.258e-5	1	NC	1	3763.117	1
461		max	.002	1	.004	2	.006	1	-2.273e-6	15	NC	1	NC	2
462		min	0	3	-.006	3	0	15	-6.258e-5	1	NC	1	4123.756	1
463		max	.002	1	.004	2	.005	1	-2.273e-6	15	NC	1	NC	2
464		min	0	3	-.006	3	0	15	-6.258e-5	1	NC	1	4556.956	1
465		max	.002	1	.004	2	.005	1	-2.273e-6	15	NC	1	NC	2
466		min	0	3	-.005	3	0	15	-6.258e-5	1	NC	1	5083.057	1
467		max	.002	1	.003	2	.004	1	-2.273e-6	15	NC	1	NC	2
468		min	0	3	-.005	3	0	15	-6.258e-5	1	NC	1	5730.267	1
469		max	.002	1	.003	2	.004	1	-2.273e-6	15	NC	1	NC	2
470		min	0	3	-.004	3	0	15	-6.258e-5	1	NC	1	6538.624	1
471		max	.002	1	.003	2	.003	1	-2.273e-6	15	NC	1	NC	2
472		min	0	3	-.004	3	0	15	-6.258e-5	1	NC	1	7566.526	1
473		max	.002	1	.003	2	.003	1	-2.273e-6	15	NC	1	NC	2
474		min	0	3	-.004	3	0	15	-6.258e-5	1	NC	1	8901.886	1
475		max	.001	1	.002	2	.002	1	-2.273e-6	15	NC	1	NC	1
476		min	0	3	-.003	3	0	15	-6.258e-5	1	NC	1	NC	1
477		max	.001	1	.002	2	.002	1	-2.273e-6	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-6.258e-5	1	NC	1	NC	1
479		max	.001	1	.002	2	.001	1	-2.273e-6	15	NC	1	NC	1
480		min	0	3	-.003	3	0	15	-6.258e-5	1	NC	1	NC	1
481		max	0	1	.002	2	.001	1	-2.273e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-6.258e-5	1	NC	1	NC	1
483		max	0	1	.001	2	0	1	-2.273e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-6.258e-5	1	NC	1	NC	1
485		max	0	1	.001	2	0	1	-2.273e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-6.258e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-2.273e-6	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-6.258e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.273e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-6.258e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.273e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.258e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.273e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.258e-5	1	NC	1	NC	1
495	M1	max	.009	3	.211	2	0	1	9.048e-3	1	NC	1	NC	1
496		min	-.005	2	-.054	3	0	15	-1.702e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.009	3	.104	2	0	15	4.367e-3	1	NC	5	NC	1
498			min	-.005	2	-.027	3	-.005	1	-8.449e-3	3	1266.902	2	NC	1
499		3	max	.009	3	.012	3	0	15	2.093e-5	10	NC	5	NC	1
500			min	-.005	2	-.01	2	-.008	1	-1.556e-4	1	613.171	2	NC	1
501		4	max	.008	3	.073	3	0	15	4.215e-3	1	NC	15	NC	1
502			min	-.005	2	-.137	2	-.007	1	-3.976e-3	3	389.855	2	NC	1
503		5	max	.008	3	.148	3	0	15	8.585e-3	1	NC	15	NC	1
504			min	-.005	2	-.268	2	-.005	1	-7.859e-3	3	282.95	2	NC	1
505		6	max	.008	3	.229	3	0	15	1.295e-2	1	7988.212	15	NC	1
506			min	-.005	2	-.394	2	-.002	1	-1.174e-2	3	223.8	2	NC	1
507		7	max	.008	3	.306	3	0	1	1.733e-2	1	6751.509	15	NC	1
508			min	-.004	2	-.507	2	0	3	-1.562e-2	3	188.768	2	NC	1
509		8	max	.008	3	.37	3	0	1	2.17e-2	1	6018.639	15	NC	1
510			min	-.004	2	-.596	2	0	15	-1.951e-2	3	168.001	2	NC	1
511		9	max	.008	3	.412	3	0	15	2.384e-2	1	5634.698	15	NC	1
512			min	-.004	2	-.652	2	0	1	-1.999e-2	3	157.014	1	NC	1
513		10	max	.007	3	.428	3	0	1	2.459e-2	2	5517.22	15	NC	1
514			min	-.004	2	-.671	2	0	15	-1.82e-2	3	153.711	1	NC	1
515		11	max	.007	3	.418	3	0	1	2.592e-2	2	5634.445	15	NC	1
516			min	-.004	2	-.652	2	0	15	-1.641e-2	3	157.263	1	NC	1
517		12	max	.007	3	.383	3	0	15	2.475e-2	2	6018.096	15	NC	1
518			min	-.004	2	-.594	2	0	1	-1.42e-2	3	168.795	1	NC	1
519		13	max	.007	3	.326	3	0	15	1.985e-2	2	6750.549	15	NC	1
520			min	-.004	2	-.502	1	0	1	-1.136e-2	3	190.99	1	NC	1
521		14	max	.007	3	.254	3	.002	1	1.495e-2	2	7986.58	15	NC	1
522			min	-.004	2	-.386	1	0	15	-8.525e-3	3	228.722	1	NC	1
523		15	max	.006	3	.172	3	.004	1	1.004e-2	2	NC	15	NC	1
524			min	-.004	2	-.258	1	0	15	-5.688e-3	3	293.117	1	NC	1
525		16	max	.006	3	.087	3	.007	1	5.142e-3	2	NC	15	NC	1
526			min	-.004	2	-.127	1	0	15	-2.851e-3	3	411.011	1	NC	1
527		17	max	.006	3	.004	3	.007	1	4.939e-4	1	NC	5	NC	1
528			min	-.004	2	-.006	2	0	15	-1.391e-5	3	659.93	1	NC	1
529		18	max	.006	3	.101	1	.005	1	6.46e-3	2	NC	5	NC	1
530			min	-.004	2	-.071	3	0	15	-2.202e-3	3	1384.091	1	NC	1
531		19	max	.006	3	.197	1	0	15	1.288e-2	2	NC	1	NC	1
532			min	-.004	2	-.141	3	0	1	-4.48e-3	3	NC	1	NC	1
533	M5	1	max	.026	3	.371	2	0	1	0	1	NC	1	NC	1
534			min	-.018	2	-.023	3	0	1	0	1	NC	1	NC	1
535		2	max	.026	3	.183	2	0	1	0	1	NC	5	NC	1
536			min	-.018	2	-.014	3	0	1	0	1	730.944	2	NC	1
537		3	max	.026	3	.036	3	0	1	0	1	NC	15	NC	1
538			min	-.018	2	-.03	2	0	1	0	1	340.839	2	NC	1
539		4	max	.025	3	.157	3	0	1	0	1	8301.035	15	NC	1
540			min	-.018	2	-.29	2	0	1	0	1	206.451	2	NC	1
541		5	max	.025	3	.33	3	0	1	0	1	5766.521	15	NC	1
542			min	-.017	2	-.575	2	0	1	0	1	143.992	2	NC	1
543		6	max	.024	3	.526	3	0	1	0	1	4415.68	15	NC	1
544			min	-.017	2	-.861	2	0	1	0	1	110.538	2	NC	1
545		7	max	.024	3	.72	3	0	1	0	1	3639.864	15	NC	1
546			min	-.016	2	-1.121	2	0	1	0	1	91.25	2	NC	1
547		8	max	.023	3	.883	3	0	1	0	1	3190.767	15	NC	1
548			min	-.016	2	-1.331	1	0	1	0	1	79.948	1	NC	1
549		9	max	.023	3	.989	3	0	1	0	1	2960.93	15	NC	1
550			min	-.016	2	-1.464	1	0	1	0	1	74.105	1	NC	1
551		10	max	.022	3	1.027	3	0	1	0	1	2891.692	15	NC	1
552			min	-.016	2	-1.509	1	0	1	0	1	72.373	1	NC	1
553		11	max	.022	3	1.002	3	0	1	0	1	2961.07	15	NC	1





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.015	2	-1.464	1	0	1	0	1	74.239	1	NC	1
555		12	max	.021	3	.914	3	0	1	0	1	3191.095	15	NC	1
556			min	-.015	2	-1.327	1	0	1	0	1	80.396	1	NC	1
557		13	max	.02	3	.772	3	0	1	0	1	3640.518	15	NC	1
558			min	-.015	2	-1.11	1	0	1	0	1	92.567	1	NC	1
559		14	max	.02	3	.594	3	0	1	0	1	4416.935	15	NC	1
560			min	-.014	2	-.84	1	0	1	0	1	113.881	1	NC	1
561		15	max	.019	3	.396	3	0	1	0	1	5768.975	15	NC	1
562			min	-.014	2	-.548	1	0	1	0	1	151.701	1	NC	1
563		16	max	.019	3	.196	3	0	1	0	1	8306.154	15	NC	1
564			min	-.014	2	-.263	1	0	1	0	1	224.485	1	NC	1
565		17	max	.018	3	.012	3	0	1	0	1	NC	15	NC	1
566			min	-.014	2	-.016	2	0	1	0	1	386.251	1	NC	1
567		18	max	.018	3	.175	1	0	1	0	1	NC	5	NC	1
568			min	-.014	2	-.143	3	0	1	0	1	855.974	1	NC	1
569		19	max	.018	3	.33	1	0	1	0	1	NC	1	NC	1
570			min	-.014	2	-.28	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.009	3	.211	2	0	15	1.702e-2	3	NC	1	NC	1
572			min	-.005	2	-.054	3	0	1	-9.048e-3	1	NC	1	NC	1
573		2	max	.009	3	.104	2	.005	1	8.449e-3	3	NC	5	NC	1
574			min	-.005	2	-.027	3	0	15	-4.367e-3	1	1266.902	2	NC	1
575		3	max	.009	3	.012	3	.008	1	1.556e-4	1	NC	5	NC	1
576			min	-.005	2	-.01	2	0	15	-2.093e-5	10	613.171	2	NC	1
577		4	max	.008	3	.073	3	.007	1	3.976e-3	3	NC	15	NC	1
578			min	-.005	2	-.137	2	0	15	-4.215e-3	1	389.855	2	NC	1
579		5	max	.008	3	.148	3	.005	1	7.859e-3	3	NC	15	NC	1
580			min	-.005	2	-.268	2	0	15	-8.585e-3	1	282.95	2	NC	1
581		6	max	.008	3	.229	3	.002	1	1.174e-2	3	7988.212	15	NC	1
582			min	-.005	2	-.394	2	0	15	-1.295e-2	1	223.8	2	NC	1
583		7	max	.008	3	.306	3	0	3	1.562e-2	3	6751.509	15	NC	1
584			min	-.004	2	-.507	2	0	1	-1.733e-2	1	188.768	2	NC	1
585		8	max	.008	3	.37	3	0	15	1.951e-2	3	6018.639	15	NC	1
586			min	-.004	2	-.596	2	0	1	-2.17e-2	1	168.001	2	NC	1
587		9	max	.008	3	.412	3	0	1	1.999e-2	3	5634.698	15	NC	1
588			min	-.004	2	-.652	2	0	15	-2.384e-2	1	157.014	1	NC	1
589		10	max	.007	3	.428	3	0	15	1.82e-2	3	5517.22	15	NC	1
590			min	-.004	2	-.671	2	0	1	-2.459e-2	2	153.711	1	NC	1
591		11	max	.007	3	.418	3	0	15	1.641e-2	3	5634.445	15	NC	1
592			min	-.004	2	-.652	2	0	1	-2.592e-2	2	157.263	1	NC	1
593		12	max	.007	3	.383	3	0	1	1.42e-2	3	6018.096	15	NC	1
594			min	-.004	2	-.594	2	0	15	-2.475e-2	2	168.795	1	NC	1
595		13	max	.007	3	.326	3	0	1	1.136e-2	3	6750.549	15	NC	1
596			min	-.004	2	-.502	1	0	15	-1.985e-2	2	190.99	1	NC	1
597		14	max	.007	3	.254	3	0	15	8.525e-3	3	7986.58	15	NC	1
598			min	-.004	2	-.386	1	-.002	1	-1.495e-2	2	228.722	1	NC	1
599		15	max	.006	3	.172	3	0	15	5.688e-3	3	NC	15	NC	1
600			min	-.004	2	-.258	1	-.004	1	-1.004e-2	2	293.117	1	NC	1
601		16	max	.006	3	.087	3	0	15	2.851e-3	3	NC	15	NC	1
602			min	-.004	2	-.127	1	-.007	1	-5.142e-3	2	411.011	1	NC	1
603		17	max	.006	3	.004	3	0	15	1.391e-5	3	NC	5	NC	1
604			min	-.004	2	-.006	2	-.007	1	-4.939e-4	1	659.93	1	NC	1
605		18	max	.006	3	.101	1	0	15	2.202e-3	3	NC	5	NC	1
606			min	-.004	2	-.071	3	-.005	1	-6.46e-3	2	1384.091	1	NC	1
607		19	max	.006	3	.197	1	0	1	4.48e-3	3	NC	1	NC	1
608			min	-.004	2	-.141	3	0	15	-1.288e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.6025.0

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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

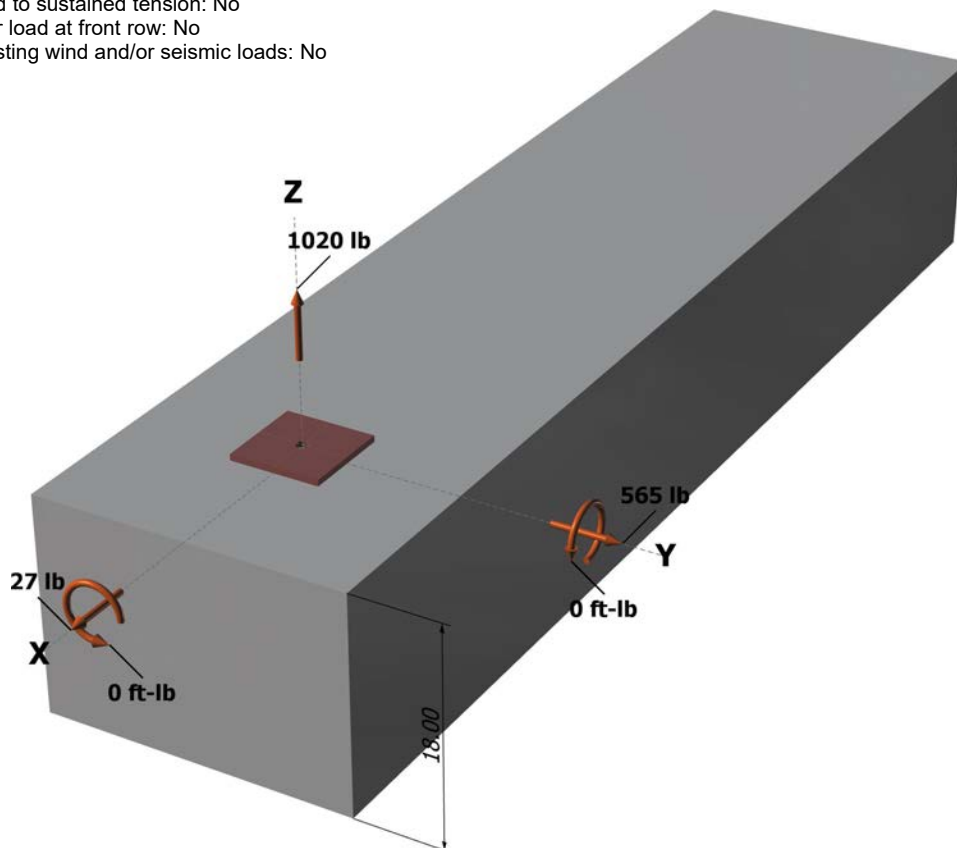
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

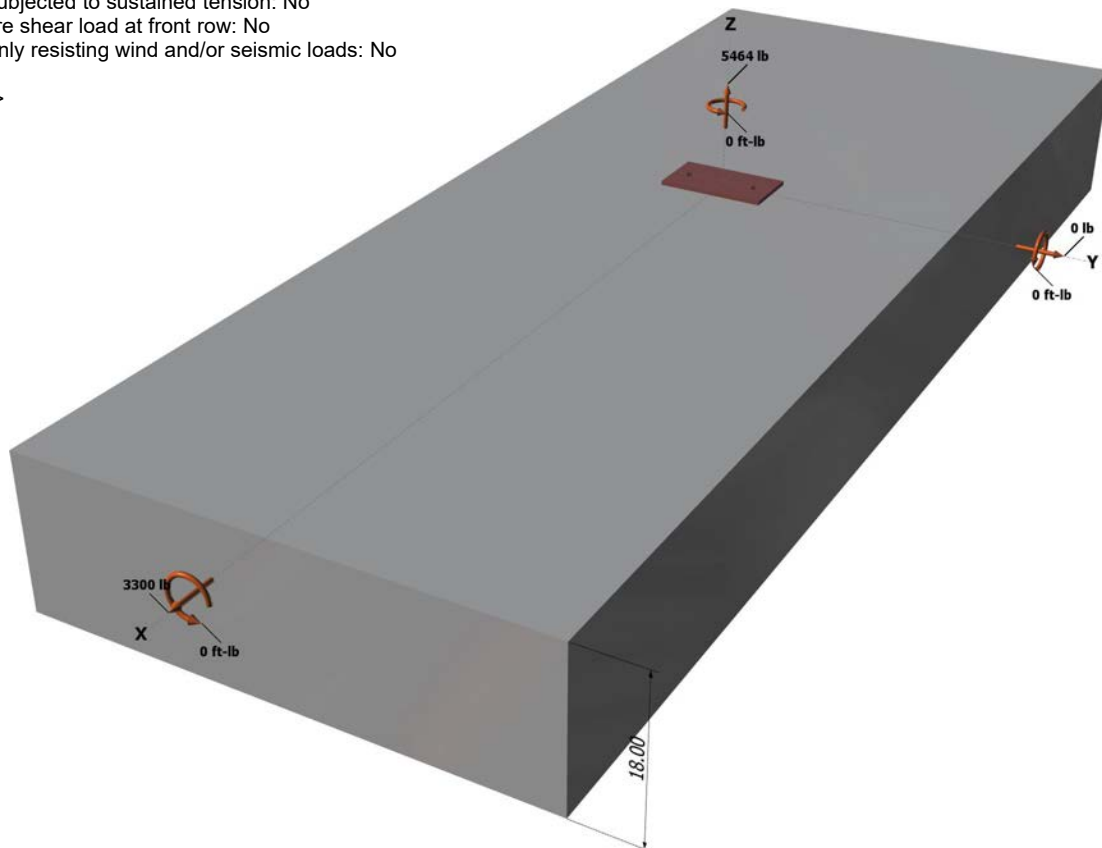
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



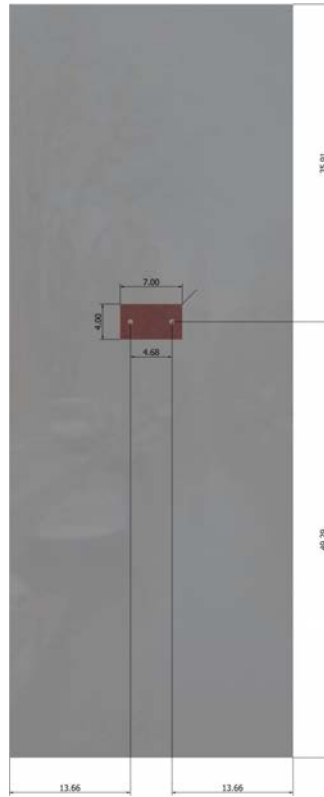
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.