

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	115 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 20.76 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

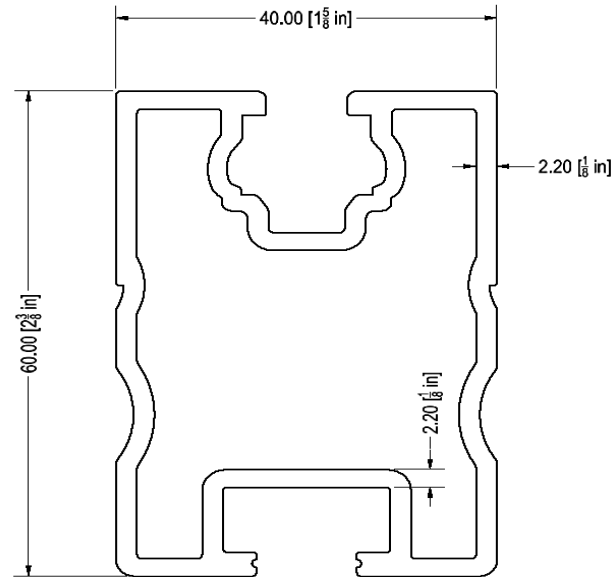
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	84 in
$\Phi F_{ty}$ STRONG-AXIS =	28.54 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.764 k-ft
$M_z$ =	0.192 k-ft
$M_{y \text{ allowable}}$ =	1.214 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>85%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.56 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.609 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.311 k
$M_{y \text{ allowable}}$ =	1.451 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>44%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.049 k-ft
$P_n$ =	0.241 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>14%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.632 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>17%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.929 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>18%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.006 k-ft
$P_n$ =	0.237 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>15%</b>



A cross brace kit is required every 13 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

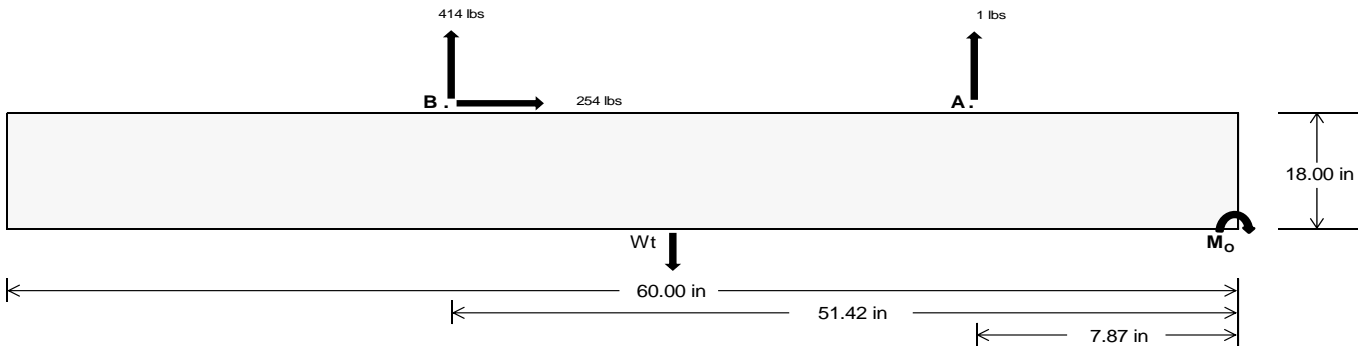
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>12.20</b>	<b>1798.72</b>	k
Compressive Load =	<b>1441.48</b>	<b>1333.61</b>	k
Lateral Load =	<b>39.83</b>	<b>1102.66</b>	k
Moment (Weak Axis) =	<b>0.06</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 25862.5$  in-lbs  
Resisting Force Required = 862.08 lbs  
S.F. = 1.67  
Weight Required = 1436.81 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 254.26 lbs  
Friction = 0.4  
Weight Required = 635.66 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 254.26 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

	Ballast Width			
	21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	552 lbs	552 lbs	552 lbs	552 lbs	418 lbs	418 lbs	418 lbs	418 lbs	678 lbs	678 lbs	678 lbs	678 lbs	-2 lbs	-2 lbs	-2 lbs	-2 lbs
$F_B$	388 lbs	388 lbs	388 lbs	388 lbs	540 lbs	540 lbs	540 lbs	540 lbs	660 lbs	660 lbs	660 lbs	660 lbs	-828 lbs	-828 lbs	-828 lbs	-828 lbs
$F_V$	67 lbs	67 lbs	67 lbs	67 lbs	464 lbs	464 lbs	464 lbs	464 lbs	393 lbs	393 lbs	393 lbs	393 lbs	-509 lbs	-509 lbs	-509 lbs	-509 lbs
$P_{total}$	2843 lbs	2934 lbs	3024 lbs	3115 lbs	2862 lbs	2953 lbs	3043 lbs	3134 lbs	3242 lbs	3332 lbs	3423 lbs	3514 lbs	312 lbs	367 lbs	421 lbs	476 lbs
$M$	426 lbs-ft	426 lbs-ft	426 lbs-ft	426 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft	503 lbs-ft	661 lbs-ft	661 lbs-ft	661 lbs-ft	661 lbs-ft	711 lbs-ft	711 lbs-ft	711 lbs-ft	711 lbs-ft
$e$	0.15 ft	0.15 ft	0.14 ft	0.14 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.28 ft	1.94 ft	1.69 ft	1.50 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	266.6 psf	264.3 psf	262.3 psf	260.4 psf	258.1 psf	256.3 psf	254.6 psf	253.0 psf	279.9 psf	277.0 psf	274.4 psf	272.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	383.3 psf	375.8 psf	368.9 psf	362.6 psf	396.0 psf	387.9 psf	380.5 psf	373.7 psf	461.1 psf	450.1 psf	439.9 psf	430.7 psf	532.2 psf	237.7 psf	180.5 psf	157.8 psf

Maximum Bearing Pressure = 532 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

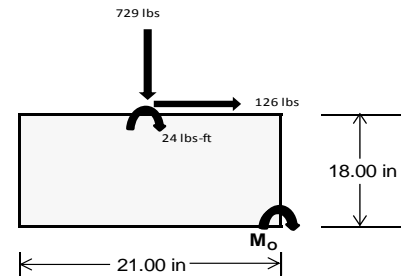
### Overturning Check

$M_o = 424.9 \text{ ft-lbs}$   
 Resisting Force Required = 485.60 lbs  
 S.F. = 1.67  
 Weight Required = 809.33 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	142 lbs	147 lbs	82 lbs	318 lbs	729 lbs	272 lbs	84 lbs	-3 lbs	27 lbs
$F_v$	21 lbs	167 lbs	22 lbs	14 lbs	126 lbs	17 lbs	22 lbs	167 lbs	22 lbs
$P_{total}$	2498 lbs	2503 lbs	2438 lbs	2561 lbs	2972 lbs	2515 lbs	773 lbs	686 lbs	716 lbs
$M$	61 lbs-ft	283 lbs-ft	66 lbs-ft	39 lbs-ft	213 lbs-ft	52 lbs-ft	62 lbs-ft	283 lbs-ft	65 lbs-ft
$e$	0.02 ft	0.11 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.08 ft	0.41 ft	0.09 ft
$L/6$	0.29 ft	1.52 ft	1.70 ft	1.72 ft	1.61 ft	1.71 ft	1.59 ft	0.93 ft	1.57 ft
$f_{min}$	261.7 sqft	175.1 sqft	252.9 sqft	277.4 sqft	256.1 sqft	267.2 sqft	63.9 sqft	-32.4 sqft	56.6 sqft
$f_{max}$	309.3 psf	397.0 psf	304.5 psf	308.0 psf	423.3 psf	307.6 psf	112.7 psf	189.2 psf	107.2 psf



Maximum Bearing Pressure = 423 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



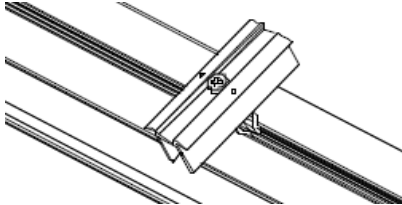
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

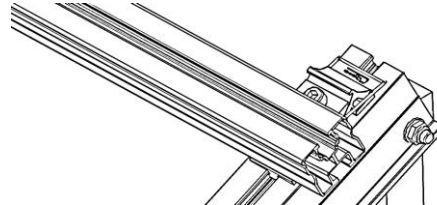
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.444 k
Allowable Uplift =	1.214 k
Utilization =	<u>37%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.099 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

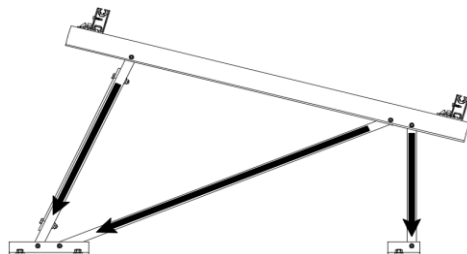
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.109 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Diagonal Strut

Maximum Axial Load =	0.632 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.178 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

#### Bracing

Maximum Axial Load =	0.237 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.103 in
	<u>0.103 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 84.00 \text{ in}$$

$$J = 0.255$$

$$218.731$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 84.00 \text{ in}$$

$$J = 0.255$$

$$227.139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.214 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.19 \\ &22.5321 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\ \phi F_L &= 29.6 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.19 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\ \phi F_L &= 29.6 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.451 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 0.16$$

$$103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	286.947	1	-.016	15	.52	1	0	10	.001	4	0	15
30			min	-360.485	3	-.073	3	-.384	5	-.001	1	0	3	0	6
31		16	max	287.073	1	-.028	15	.52	1	0	10	.001	4	0	15
32			min	-360.391	3	-.115	4	-.498	5	-.001	1	0	3	0	6
33		17	max	287.199	1	-.04	15	.52	1	0	10	.001	1	0	15
34			min	-360.296	3	-.166	4	-.613	5	-.001	1	0	3	0	6
35		18	max	287.325	1	-.052	15	.52	1	0	10	.001	1	0	15
36			min	-360.202	3	-.218	4	-.727	5	-.001	1	0	3	0	6
37		19	max	287.451	1	-.064	15	.52	1	0	10	.001	1	0	15
38			min	-360.107	3	-.269	4	-.842	5	-.001	1	0	3	0	6
39	M3	1	max	155.621	2	1.756	6	-.034	12	0	5	.002	1	0	6
40			min	-174.058	3	.412	15	-1.42	4	0	1	0	12	0	15
41		2	max	155.552	2	1.579	6	-.034	12	0	5	.002	1	0	2
42			min	-174.11	3	.371	15	-1.286	4	0	1	0	12	0	12
43		3	max	155.483	2	1.402	6	-.034	12	0	5	.002	1	0	2
44			min	-174.162	3	.329	15	-1.152	4	0	1	0	15	0	3
45		4	max	155.413	2	1.225	6	-.034	12	0	5	.001	1	0	15
46			min	-174.214	3	.287	15	-1.019	4	0	1	0	5	0	4
47		5	max	155.344	2	1.048	6	-.034	12	0	5	.001	1	0	15
48			min	-174.266	3	.246	15	-.885	4	0	1	0	5	0	4
49		6	max	155.275	2	.871	6	-.034	12	0	5	.001	1	0	15
50			min	-174.318	3	.204	15	-.751	4	0	1	0	5	0	4
51		7	max	155.205	2	.695	6	-.034	12	0	5	.001	1	0	15
52			min	-174.37	3	.163	15	-.618	4	0	1	0	5	0	4
53		8	max	155.136	2	.518	6	-.034	12	0	5	0	1	0	15
54			min	-174.422	3	.121	15	-.557	1	0	1	0	5	-.001	4
55		9	max	155.067	2	.341	6	-.034	12	0	5	0	1	0	15
56			min	-174.474	3	.08	15	-.557	1	0	1	0	5	-.001	4
57		10	max	154.997	2	.164	6	-.034	12	0	5	0	1	0	15
58			min	-174.526	3	.038	15	-.557	1	0	1	0	5	-.001	4
59		11	max	154.928	2	.016	2	.038	5	0	5	0	1	0	15
60			min	-174.578	3	-.038	3	-.557	1	0	1	0	5	-.001	4
61		12	max	154.859	2	-.045	15	.172	5	0	5	0	1	0	15
62			min	-174.63	3	-.19	4	-.557	1	0	1	0	5	-.001	4
63		13	max	154.789	2	-.087	15	.305	5	0	5	0	1	0	15
64			min	-174.682	3	-.367	4	-.557	1	0	1	0	5	-.001	4
65		14	max	154.72	2	-.128	15	.439	5	0	5	0	1	0	15
66			min	-174.734	3	-.543	4	-.557	1	0	1	0	5	-.001	4
67		15	max	154.651	2	-.17	15	.573	5	0	5	0	1	0	15
68			min	-174.786	3	-.72	4	-.557	1	0	1	0	5	0	4
69		16	max	154.581	2	-.211	15	.706	5	0	5	0	1	0	15
70			min	-174.838	3	-.897	4	-.557	1	0	1	0	5	0	4
71		17	max	154.512	2	-.253	15	.84	5	0	5	0	12	0	15
72			min	-174.89	3	-1.074	4	-.557	1	0	1	0	4	0	4
73		18	max	154.443	2	-.294	15	.974	5	0	5	0	12	0	15
74			min	-174.942	3	-1.251	4	-.557	1	0	1	0	1	0	4
75		19	max	154.373	2	-.336	15	1.107	5	0	5	0	5	0	1
76			min	-174.994	3	-1.428	4	-.557	1	0	1	0	1	0	1
77	M4	1	max	422.74	1	0	1	-.191	12	0	1	0	5	0	1
78			min	5.611	12	0	1	-29.94	4	0	1	0	2	0	1
79		2	max	422.805	1	0	1	-.191	12	0	1	0	12	0	1
80			min	5.644	12	0	1	-29.996	4	0	1	-.003	4	0	1
81		3	max	422.87	1	0	1	-.191	12	0	1	0	12	0	1
82			min	5.676	12	0	1	-30.053	4	0	1	-.005	4	0	1
83		4	max	422.934	1	0	1	-.191	12	0	1	0	12	0	1
84			min	5.708	12	0	1	-30.109	4	0	1	-.008	4	0	1
85		5	max	422.999	1	0	1	-.191	12	0	1	0	12	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	5.741	12	0	1	-30.165	4	0	1	-.011	4	0	1
87	6	max	423.064	1	0	1	-.191	12	0	1	0	12	0	1
88		min	5.773	12	0	1	-30.221	4	0	1	-.013	4	0	1
89	7	max	423.129	1	0	1	-.191	12	0	1	0	12	0	1
90		min	5.805	12	0	1	-30.277	4	0	1	-.016	4	0	1
91	8	max	423.193	1	0	1	-.191	12	0	1	0	12	0	1
92		min	5.838	12	0	1	-30.333	4	0	1	-.019	4	0	1
93	9	max	423.258	1	0	1	-.191	12	0	1	0	12	0	1
94		min	5.87	12	0	1	-30.389	4	0	1	-.022	4	0	1
95	10	max	423.323	1	0	1	-.191	12	0	1	0	12	0	1
96		min	5.902	12	0	1	-30.445	4	0	1	-.024	4	0	1
97	11	max	423.387	1	0	1	-.191	12	0	1	0	12	0	1
98		min	5.935	12	0	1	-30.501	4	0	1	-.027	4	0	1
99	12	max	423.452	1	0	1	-.191	12	0	1	0	12	0	1
100		min	5.967	12	0	1	-30.557	4	0	1	-.03	4	0	1
101	13	max	423.517	1	0	1	-.191	12	0	1	0	12	0	1
102		min	5.999	12	0	1	-30.613	4	0	1	-.032	4	0	1
103	14	max	423.581	1	0	1	-.191	12	0	1	0	12	0	1
104		min	6.032	12	0	1	-30.669	4	0	1	-.035	4	0	1
105	15	max	423.646	1	0	1	-.191	12	0	1	0	12	0	1
106		min	6.064	12	0	1	-30.725	4	0	1	-.038	4	0	1
107	16	max	423.711	1	0	1	-.191	12	0	1	0	12	0	1
108		min	6.097	12	0	1	-30.782	4	0	1	-.041	4	0	1
109	17	max	423.776	1	0	1	-.191	12	0	1	0	12	0	1
110		min	6.129	12	0	1	-30.838	4	0	1	-.043	4	0	1
111	18	max	423.84	1	0	1	-.191	12	0	1	0	12	0	1
112		min	6.161	12	0	1	-30.894	4	0	1	-.046	4	0	1
113	19	max	423.905	1	0	1	-.191	12	0	1	0	12	0	1
114		min	6.194	12	0	1	-30.95	4	0	1	-.049	4	0	1
115	M6	1	max	926.718	1	.642	6	1.231	4	0	0	3	0	1
116		min	-1177.702	3	.144	15	-.15	3	0	5	0	1	0	1
117	2	max	926.844	1	.591	6	1.117	4	0	1	0	4	0	15
118		min	-1177.607	3	.132	15	-.15	3	0	5	0	11	0	6
119	3	max	926.97	1	.54	6	1.002	4	0	1	0	4	0	15
120		min	-1177.513	3	.12	15	-.15	3	0	5	0	11	0	6
121	4	max	927.096	1	.49	2	.888	4	0	1	0	4	0	15
122		min	-1177.418	3	.108	15	-.15	3	0	5	0	10	0	6
123	5	max	927.222	1	.45	2	.773	4	0	1	0	4	0	15
124		min	-1177.324	3	.096	15	-.15	3	0	5	0	10	0	6
125	6	max	927.348	1	.41	2	.659	4	0	1	0	4	0	15
126		min	-1177.23	3	.084	15	-.15	3	0	5	0	10	0	6
127	7	max	927.473	1	.371	2	.545	4	0	1	.001	4	0	15
128		min	-1177.135	3	.065	12	-.15	3	0	5	0	3	0	2
129	8	max	927.599	1	.331	2	.43	4	0	1	.001	4	0	15
130		min	-1177.041	3	.046	12	-.15	3	0	5	0	3	0	2
131	9	max	927.725	1	.291	2	.316	4	0	1	.001	4	0	15
132		min	-1176.946	3	.026	12	-.15	3	0	5	0	3	0	2
133	10	max	927.851	1	.251	2	.201	4	0	1	.001	4	0	15
134		min	-1176.852	3	-.003	3	-.15	3	0	5	0	3	0	2
135	11	max	927.977	1	.211	2	.169	1	0	1	.001	4	0	15
136		min	-1176.758	3	-.033	3	-.15	3	0	5	0	3	0	2
137	12	max	928.103	1	.171	2	.169	1	0	1	.001	4	0	12
138		min	-1176.663	3	-.063	3	-.15	3	0	5	0	3	0	2
139	13	max	928.229	1	.131	2	.169	1	0	1	.001	4	0	12
140		min	-1176.569	3	-.093	3	-.194	5	0	5	0	3	0	2
141	14	max	928.355	1	.092	2	.169	1	0	1	.001	4	0	12
142		min	-1176.474	3	-.123	3	-.309	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	928.48	1	.052	2	.169	1	0	1	.001	4	0	12
144		min	-1176.38	3	-.153	3	-.423	5	0	5	0	3	0	2
145	16	max	928.606	1	.012	2	.169	1	0	1	.001	4	0	12
146		min	-1176.286	3	-.183	3	-.538	5	0	5	0	3	0	2
147	17	max	928.732	1	-.028	2	.169	1	0	1	.001	4	0	12
148		min	-1176.191	3	-.212	3	-.652	5	0	5	0	3	0	2
149	18	max	928.858	1	-.061	15	.169	1	0	1	0	4	0	3
150		min	-1176.097	3	-.242	3	-.766	5	0	5	0	3	0	2
151	19	max	928.984	1	-.073	15	.169	1	0	1	0	4	0	3
152		min	-1176.002	3	-.279	4	-.881	5	0	5	0	3	0	2
153	M7	1	max	632.096	2	1.774	.017	3	0	14	0	4	0	2
154		min	-542.627	3	.422	15	-1.325	5	0	3	0	3	0	3
155	2	max	632.027	2	1.597	4	.017	3	0	14	0	4	0	2
156		min	-542.679	3	.381	15	-1.192	5	0	3	0	3	0	3
157	3	max	631.958	2	1.42	4	.017	3	0	14	0	2	0	2
158		min	-542.731	3	.339	15	-1.058	5	0	3	0	3	0	3
159	4	max	631.888	2	1.243	4	.017	3	0	14	0	2	0	2
160		min	-542.783	3	.298	15	-.924	5	0	3	0	5	0	3
161	5	max	631.819	2	1.066	4	.017	3	0	14	0	2	0	15
162		min	-542.835	3	.256	15	-.791	5	0	3	0	5	0	3
163	6	max	631.75	2	.889	4	.017	3	0	14	0	2	0	15
164		min	-542.887	3	.214	15	-.657	5	0	3	0	5	0	3
165	7	max	631.68	2	.713	4	.017	3	0	14	0	2	0	15
166		min	-542.939	3	.173	15	-.523	5	0	3	0	5	0	6
167	8	max	631.611	2	.536	4	.017	3	0	14	0	2	0	15
168		min	-542.991	3	.131	15	-.39	5	0	3	0	5	-.001	6
169	9	max	631.542	2	.359	2	.017	3	0	14	0	2	0	15
170		min	-543.043	3	.07	12	-.256	5	0	3	0	5	-.001	6
171	10	max	631.472	2	.222	2	.017	3	0	14	0	2	0	15
172		min	-543.095	3	-.009	3	-.122	5	0	3	0	5	-.001	6
173	11	max	631.403	2	.084	2	.017	3	0	14	0	2	0	15
174		min	-543.147	3	-.113	3	-.005	10	0	3	0	5	-.001	6
175	12	max	631.334	2	-.035	15	.147	4	0	14	0	2	0	15
176		min	-543.199	3	-.216	3	-.005	10	0	3	0	5	-.001	6
177	13	max	631.264	2	-.077	15	.281	4	0	14	0	2	0	15
178		min	-543.251	3	-.349	6	-.005	10	0	3	0	5	-.001	6
179	14	max	631.195	2	-.118	15	.415	4	0	14	0	2	0	15
180		min	-543.303	3	-.526	6	-.005	10	0	3	0	5	-.001	6
181	15	max	631.126	2	-.16	15	.548	4	0	14	0	2	0	15
182		min	-543.355	3	-.703	6	-.005	10	0	3	0	5	0	6
183	16	max	631.056	2	-.201	15	.682	4	0	14	0	2	0	15
184		min	-543.407	3	-.879	6	-.005	10	0	3	0	5	0	6
185	17	max	630.987	2	-.243	15	.816	4	0	14	0	2	0	15
186		min	-543.459	3	-1.056	6	-.005	10	0	3	0	5	0	6
187	18	max	630.918	2	-.284	15	.949	4	0	14	0	2	0	15
188		min	-543.511	3	-1.233	6	-.005	10	0	3	0	5	0	6
189	19	max	630.848	2	-.326	15	1.083	4	0	14	0	14	0	1
190		min	-543.563	3	-1.41	6	-.005	10	0	3	0	3	0	1
191	M8	1	max	1107.664	1	0	.771	1	0	1	0	4	0	1
192		min	-10.257	3	0	1	-29.993	4	0	1	0	1	0	1
193	2	max	1107.728	1	0	1	.771	1	0	1	0	1	0	1
194		min	-10.208	3	0	1	-30.049	4	0	1	-.003	4	0	1
195	3	max	1107.793	1	0	1	.771	1	0	1	0	1	0	1
196		min	-10.16	3	0	1	-30.105	4	0	1	-.005	4	0	1
197	4	max	1107.858	1	0	1	.771	1	0	1	0	1	0	1
198		min	-10.111	3	0	1	-30.161	4	0	1	-.008	4	0	1
199	5	max	1107.923	1	0	1	.771	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-10.062	3	0	1	-30.217	4	0	1	-.011	4	0	1
201		6	max	1107.987	1	0	1	.771	1	0	1	0	1	0	1
202			min	-10.014	3	0	1	-30.274	4	0	1	-.013	4	0	1
203		7	max	1108.052	1	0	1	.771	1	0	1	0	1	0	1
204			min	-9.965	3	0	1	-30.33	4	0	1	-.016	4	0	1
205		8	max	1108.117	1	0	1	.771	1	0	1	0	1	0	1
206			min	-9.917	3	0	1	-30.386	4	0	1	-.019	4	0	1
207		9	max	1108.181	1	0	1	.771	1	0	1	0	1	0	1
208			min	-9.868	3	0	1	-30.442	4	0	1	-.022	4	0	1
209		10	max	1108.246	1	0	1	.771	1	0	1	0	1	0	1
210			min	-9.82	3	0	1	-30.498	4	0	1	-.024	4	0	1
211		11	max	1108.311	1	0	1	.771	1	0	1	0	1	0	1
212			min	-9.771	3	0	1	-30.554	4	0	1	-.027	4	0	1
213		12	max	1108.376	1	0	1	.771	1	0	1	0	1	0	1
214			min	-9.723	3	0	1	-30.61	4	0	1	-.03	4	0	1
215		13	max	1108.44	1	0	1	.771	1	0	1	0	1	0	1
216			min	-9.674	3	0	1	-30.666	4	0	1	-.033	4	0	1
217		14	max	1108.505	1	0	1	.771	1	0	1	0	1	0	1
218			min	-9.626	3	0	1	-30.722	4	0	1	-.035	4	0	1
219		15	max	1108.57	1	0	1	.771	1	0	1	0	1	0	1
220			min	-9.577	3	0	1	-30.778	4	0	1	-.038	4	0	1
221		16	max	1108.634	1	0	1	.771	1	0	1	.001	1	0	1
222			min	-9.529	3	0	1	-30.834	4	0	1	-.041	4	0	1
223		17	max	1108.699	1	0	1	.771	1	0	1	.001	1	0	1
224			min	-9.48	3	0	1	-30.89	4	0	1	-.044	4	0	1
225		18	max	1108.764	1	0	1	.771	1	0	1	.001	1	0	1
226			min	-9.432	3	0	1	-30.947	4	0	1	-.046	4	0	1
227		19	max	1108.828	1	0	1	.771	1	0	1	.001	1	0	1
228			min	-9.383	3	0	1	-31.003	4	0	1	-.049	4	0	1
229	M10	1	max	296.298	1	.681	4	1.405	5	.001	1	0	1	0	1
230			min	-337.641	3	.172	15	-.184	1	-.002	5	0	5	0	1
231		2	max	296.423	1	.629	4	1.29	5	.001	1	0	1	0	15
232			min	-337.546	3	.16	15	-.184	1	-.002	5	0	3	0	4
233		3	max	296.549	1	.578	4	1.176	5	.001	1	0	4	0	15
234			min	-337.452	3	.148	15	-.184	1	-.002	5	0	3	0	4
235		4	max	296.675	1	.527	4	1.061	5	.001	1	0	4	0	15
236			min	-337.358	3	.136	15	-.184	1	-.002	5	0	3	0	4
237		5	max	296.801	1	.476	4	.947	5	.001	1	0	4	0	15
238			min	-337.263	3	.124	15	-.184	1	-.002	5	0	3	0	4
239		6	max	296.927	1	.425	4	.833	5	.001	1	0	4	0	15
240			min	-337.169	3	.112	15	-.184	1	-.002	5	0	3	0	4
241		7	max	297.053	1	.374	4	.718	5	.001	1	.001	4	0	15
242			min	-337.074	3	.1	15	-.184	1	-.002	5	0	3	0	4
243		8	max	297.179	1	.323	4	.604	5	.001	1	.001	4	0	15
244			min	-336.98	3	.088	15	-.184	1	-.002	5	0	3	0	4
245		9	max	297.305	1	.271	4	.489	5	.001	1	.001	4	0	15
246			min	-336.886	3	.076	15	-.184	1	-.002	5	0	3	0	4
247		10	max	297.43	1	.22	4	.375	5	.001	1	.001	4	0	15
248			min	-336.791	3	.064	15	-.184	1	-.002	5	0	3	0	4
249		11	max	297.556	1	.169	4	.261	5	.001	1	.001	4	0	15
250			min	-336.697	3	.05	12	-.184	1	-.002	5	0	3	0	4
251		12	max	297.682	1	.118	4	.146	5	.001	1	.001	4	0	15
252			min	-336.602	3	.03	12	-.184	1	-.002	5	0	3	0	4
253		13	max	297.808	1	.067	4	.032	5	.001	1	.001	4	0	15
254			min	-336.508	3	.008	9	-.184	1	-.002	5	0	3	0	4
255		14	max	297.934	1	.023	5	-.004	12	.001	1	.001	4	0	15
256			min	-336.414	3	-.031	1	-.184	1	-.002	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	298.06	1	.005	5	-.004	12	.001	1	.001	4	0	15
258			min	-336.319	3	-.071	1	-.226	4	-.002	5	0	1	0	4
259		16	max	298.186	1	-.008	15	-.004	12	.001	1	.001	5	0	15
260			min	-336.225	3	-.111	1	-.34	4	-.002	5	0	1	0	4
261		17	max	298.311	1	-.02	15	-.004	12	.001	1	.001	5	0	15
262			min	-336.13	3	-.151	1	-.455	4	-.002	5	0	1	0	4
263		18	max	298.437	1	-.032	15	-.004	12	.001	1	.001	5	0	15
264			min	-336.036	3	-.191	1	-.569	4	-.002	5	0	1	0	4
265		19	max	298.563	1	-.044	15	-.004	12	.001	1	.001	5	0	15
266			min	-335.942	3	-.241	6	-.683	4	-.002	5	0	1	0	4
267	M11	1	max	155.345	2	1.746	6	.639	1	.002	4	.001	5	0	1
268			min	-174.693	3	.406	15	-1.198	5	0	10	-.002	1	0	15
269		2	max	155.276	2	1.57	6	.639	1	.002	4	0	5	0	1
270			min	-174.745	3	.364	15	-1.064	5	0	10	-.002	1	0	3
271		3	max	155.207	2	1.393	6	.639	1	.002	4	0	5	0	1
272			min	-174.797	3	.323	15	-.93	5	0	10	-.002	1	0	3
273		4	max	155.137	2	1.216	6	.639	1	.002	4	0	5	0	15
274			min	-174.849	3	.281	15	-.797	5	0	10	-.001	1	0	4
275		5	max	155.068	2	1.039	6	.639	1	.002	4	0	3	0	15
276			min	-174.901	3	.239	15	-.663	5	0	10	-.001	1	0	4
277		6	max	154.999	2	.862	6	.639	1	.002	4	0	3	0	15
278			min	-174.953	3	.198	15	-.529	5	0	10	-.001	1	0	4
279		7	max	154.929	2	.685	6	.639	1	.002	4	0	3	0	15
280			min	-175.005	3	.156	15	-.396	5	0	10	-.001	1	0	4
281		8	max	154.86	2	.509	6	.639	1	.002	4	0	3	0	15
282			min	-175.057	3	.115	15	-.262	5	0	10	0	1	-.001	4
283		9	max	154.791	2	.332	6	.639	1	.002	4	0	3	0	15
284			min	-175.109	3	.073	15	-.128	5	0	10	0	1	-.001	4
285		10	max	154.721	2	.155	1	.639	1	.002	4	0	3	0	15
286			min	-175.161	3	.032	15	-.004	3	0	10	0	1	-.001	4
287		11	max	154.652	2	.017	1	.639	1	.002	4	0	3	0	15
288			min	-175.213	3	-.056	3	-.004	3	0	10	0	1	-.001	4
289		12	max	154.583	2	-.052	15	.639	1	.002	4	0	3	0	15
290			min	-175.265	3	-.199	4	-.004	3	0	10	0	1	-.001	4
291		13	max	154.513	2	-.093	15	.639	1	.002	4	0	3	0	15
292			min	-175.317	3	-.376	4	-.004	3	0	10	0	1	-.001	4
293		14	max	154.444	2	-.135	15	.675	4	.002	4	0	3	0	15
294			min	-175.369	3	-.553	4	-.004	3	0	10	0	1	-.001	4
295		15	max	154.375	2	-.176	15	.809	4	.002	4	0	4	0	15
296			min	-175.421	3	-.73	4	-.004	3	0	10	0	10	0	4
297		16	max	154.305	2	-.218	15	.942	4	.002	4	0	4	0	15
298			min	-175.473	3	-.907	4	-.004	3	0	10	0	10	0	4
299		17	max	154.236	2	-.259	15	1.076	4	.002	4	0	4	0	15
300			min	-175.525	3	-1.083	4	-.004	3	0	10	0	10	0	4
301		18	max	154.167	2	-.301	15	1.21	4	.002	4	0	4	0	15
302			min	-175.577	3	-1.26	4	-.004	3	0	10	0	10	0	4
303		19	max	154.097	2	-.342	15	1.343	4	.002	4	.001	4	0	1
304			min	-175.629	3	-1.437	4	-.004	3	0	10	0	10	0	1
305	M12	1	max	422.392	1	0	1	3.625	1	0	1	0	4	0	1
306			min	5.966	12	0	1	-27.434	5	0	1	0	3	0	1
307		2	max	422.457	1	0	1	3.625	1	0	1	0	1	0	1
308			min	5.998	12	0	1	-27.49	5	0	1	-.002	5	0	1
309		3	max	422.522	1	0	1	3.625	1	0	1	0	1	0	1
310			min	6.03	12	0	1	-27.546	5	0	1	-.005	5	0	1
311		4	max	422.586	1	0	1	3.625	1	0	1	.001	1	0	1
312			min	6.063	12	0	1	-27.602	5	0	1	-.007	5	0	1
313		5	max	422.651	1	0	1	3.625	1	0	1	.001	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	89.503	3	4.43	9	-3.747	12	0	12	-.003	12	.174	2
372			min	-7.697	10	-25.98	2	-71.753	1	0	1	-.077	1	-.108	3
373		16	max	93.496	2	102.58	2	-3.783	12	0	1	-.004	12	.178	2
374			min	-5.756	3	-162.581	3	-72.218	1	0	5	-.093	1	-.104	3
375		17	max	93.636	2	102.338	2	-3.783	12	0	1	-.005	12	.156	2
376			min	-5.652	3	-162.762	3	-72.218	1	0	5	-.109	1	-.068	3
377		18	max	-5.9	12	361.549	2	-3.974	12	0	3	-.006	12	.079	2
378			min	-141.562	1	-157.122	3	-74.047	1	0	2	-.125	1	-.034	3
379		19	max	-5.83	12	361.307	2	-3.974	12	0	3	-.007	12	0	2
380			min	-141.422	1	-157.303	3	-74.047	1	0	2	-.141	1	0	3
381	M5	1	max	311.501	1	1124.251	3	-.049	10	0	1	.041	4	0	3
382			min	9.643	15	-935.269	1	-43.67	3	0	5	0	10	0	1
383		2	max	311.641	1	1124.069	3	-.049	10	0	1	.035	4	.202	1
384			min	9.685	15	-935.511	1	-43.67	3	0	5	-.005	3	-.243	3
385		3	max	274.23	3	6.274	9	5.028	3	0	3	.03	4	.402	1
386			min	-43.969	10	-87.733	2	-22.297	4	0	4	-.014	3	-.482	3
387		4	max	274.335	3	6.073	9	5.028	3	0	3	.025	4	.41	1
388			min	-43.852	10	-87.975	2	-22.055	4	0	4	-.013	3	-.471	3
389		5	max	274.44	3	5.871	9	5.028	3	0	3	.02	4	.418	1
390			min	-43.736	10	-88.217	2	-21.813	4	0	4	-.012	3	-.46	3
391		6	max	274.545	3	5.67	9	5.028	3	0	3	.015	4	.426	1
392			min	-43.62	10	-88.459	2	-21.571	4	0	4	-.01	3	-.449	3
393		7	max	274.649	3	5.468	9	5.028	3	0	3	.011	4	.438	2
394			min	-43.503	10	-88.701	2	-21.329	4	0	4	-.009	3	-.438	3
395		8	max	274.754	3	5.267	9	5.028	3	0	3	.006	4	.457	2
396			min	-43.387	10	-88.942	2	-21.087	4	0	4	-.008	3	-.427	3
397		9	max	274.859	3	5.065	9	5.028	3	0	3	.002	5	.477	2
398			min	-43.27	10	-89.184	2	-20.845	4	0	4	-.007	3	-.416	3
399		10	max	274.963	3	4.864	9	5.028	3	0	3	0	10	.496	2
400			min	-43.154	10	-89.426	2	-20.603	4	0	4	-.006	3	-.405	3
401		11	max	275.068	3	4.662	9	5.028	3	0	3	0	10	.515	2
402			min	-43.038	10	-89.668	2	-20.361	4	0	4	-.007	4	-.394	3
403		12	max	275.173	3	4.461	9	5.028	3	0	3	0	10	.535	2
404			min	-42.921	10	-89.91	2	-20.119	4	0	4	-.012	4	-.383	3
405		13	max	275.278	3	4.259	9	5.028	3	0	3	0	10	.554	2
406			min	-42.805	10	-90.152	2	-19.877	4	0	4	-.016	4	-.372	3
407		14	max	275.382	3	4.058	9	5.028	3	0	3	0	10	.574	2
408			min	-42.689	10	-90.393	2	-19.635	4	0	4	-.02	4	-.36	3
409		15	max	275.487	3	3.856	9	5.028	3	0	3	0	10	.594	2
410			min	-42.572	10	-90.635	2	-19.393	4	0	4	-.025	4	-.349	3
411		16	max	311.309	2	439.991	2	5.001	3	0	1	0	3	.609	2
412			min	-22.266	3	-507.155	3	-18.081	4	0	4	-.029	4	-.334	3
413		17	max	311.449	2	439.749	2	5.001	3	0	1	.001	3	.513	2
414			min	-22.161	3	-507.336	3	-17.839	4	0	4	-.033	4	-.224	3
415		18	max	-11.386	12	1191.242	2	4.574	3	0	4	.002	3	.258	2
416			min	-312.173	1	-516.437	3	-46.563	5	0	1	-.043	4	-.112	3
417		19	max	-11.316	12	1191	2	4.574	3	0	4	.003	3	0	3
418			min	-312.033	1	-516.618	3	-46.321	5	0	1	-.053	4	0	2
419	M9	1	max	141.192	1	340.127	3	197.406	4	0	3	-.002	15	0	1
420			min	3.761	15	-282.653	1	7.111	10	0	1	-.141	1	0	3
421		2	max	141.331	1	339.946	3	197.648	4	0	3	.036	5	.062	1
422			min	3.803	15	-282.894	1	7.111	10	0	1	-.122	1	-.074	3
423		3	max	88.45	3	6.825	9	68.467	1	0	1	.073	5	.122	1
424			min	-8.589	10	-23.089	2	-28.338	5	0	5	-.101	1	-.146	3
425		4	max	88.554	3	6.624	9	68.467	1	0	1	.067	5	.123	1
426			min	-8.472	10	-23.331	2	-28.096	5	0	5	-.087	1	-.143	3
427		5	max	88.659	3	6.422	9	68.467	1	0	1	.061	5	.124	1





Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-8.356	10	-23.573	2	-27.854	5	0	5	-.072	1	-.14	3
429	6	max	88.764	3	6.221	9	68.467	1	0	1	.055	5	.125	2
430		min	-8.24	10	-23.814	2	-27.612	5	0	5	-.057	1	-.137	3
431	7	max	88.869	3	6.019	9	68.467	1	0	1	.049	5	.13	2
432		min	-8.123	10	-24.056	2	-27.37	5	0	5	-.042	1	-.134	3
433	8	max	88.973	3	5.818	9	68.467	1	0	1	.043	5	.135	2
434		min	-8.007	10	-24.298	2	-27.128	5	0	5	-.027	1	-.131	3
435	9	max	89.078	3	5.616	9	68.467	1	0	1	.037	5	.141	2
436		min	-7.891	10	-24.54	2	-26.886	5	0	5	-.012	1	-.128	3
437	10	max	89.183	3	5.415	9	68.467	1	0	1	.031	4	.146	2
438		min	-7.774	10	-24.782	2	-26.644	5	0	5	0	2	-.125	3
439	11	max	89.287	3	5.213	9	68.467	1	0	1	.029	4	.152	2
440		min	-7.658	10	-25.024	2	-26.402	5	0	5	.001	10	-.122	3
441	12	max	89.392	3	5.011	9	68.467	1	0	1	.032	1	.157	2
442		min	-7.542	10	-25.265	2	-26.16	5	0	5	.003	10	-.118	3
443	13	max	89.497	3	4.81	9	68.467	1	0	1	.047	1	.163	2
444		min	-7.425	10	-25.507	2	-25.918	5	0	5	.005	10	-.115	3
445	14	max	89.602	3	4.608	9	68.467	1	0	1	.062	1	.168	2
446		min	-7.309	10	-25.749	2	-25.676	5	0	5	.005	12	-.112	3
447	15	max	89.706	3	4.407	9	68.467	1	0	1	.077	1	.174	2
448		min	-7.192	10	-25.991	2	-25.434	5	0	5	.002	15	-.108	3
449	16	max	93.804	2	102.339	2	69.015	1	0	10	.093	1	.178	2
450		min	-5.886	3	-163.05	3	-23.989	5	0	4	0	5	-.104	3
451	17	max	93.943	2	102.098	2	69.015	1	0	10	.108	1	.156	2
452		min	-5.782	3	-163.231	3	-23.747	5	0	4	-.006	5	-.068	3
453	18	max	.497	15	361.549	2	72.713	1	0	2	.123	1	.079	2
454		min	-141.199	1	-157.118	3	-50.86	5	0	3	-.017	5	-.034	3
455	19	max	.539	15	361.308	2	72.713	1	0	2	.139	1	0	2
456		min	-141.059	1	-157.299	3	-50.618	5	0	3	-.028	5	0	3
457	M13	1	max	197.421	4	282.218	1	-3.761	15	0	.141	1	0	1
458		min	7.114	10	-340.124	3	-141.174	1	0	3	.002	15	0	3
459	2	max	189.782	4	199.114	1	-2.431	15	0	1	.044	1	.226	3
460		min	7.114	10	-239.886	3	-108.145	1	0	3	0	5	-.187	1
461	3	max	182.143	4	116.01	1	-1.102	15	0	1	.003	3	.373	3
462		min	7.114	10	-139.649	3	-75.116	1	0	3	-.027	1	-.31	1
463	4	max	174.504	4	32.905	1	.242	5	0	1	0	3	.443	3
464		min	7.114	10	-39.412	3	-42.088	1	0	3	-.073	1	-.368	1
465	5	max	166.865	4	60.826	3	2.298	5	0	1	-.001	15	.434	3
466		min	7.114	10	-50.199	1	-9.059	1	0	3	-.093	1	-.361	1
467	6	max	159.226	4	161.063	3	23.97	1	0	1	0	5	.348	3
468		min	7.114	10	-133.303	1	.073	3	0	3	-.087	1	-.29	1
469	7	max	151.587	4	261.3	3	56.998	1	0	1	.005	5	.184	3
470		min	7.114	10	-216.407	1	1.425	12	0	3	-.056	1	-.154	1
471	8	max	143.948	4	361.538	3	90.027	1	0	1	.011	4	.047	1
472		min	7.114	10	-299.511	1	2.715	12	0	3	0	3	-.058	3
473	9	max	136.309	4	461.775	3	123.055	1	0	1	.084	1	.312	1
474		min	7.114	10	-382.615	1	4.004	12	0	3	.002	12	-.379	3
475	10	max	128.67	4	562.013	3	156.084	1	0	2	.193	1	.642	1
476		min	7.114	10	-465.72	1	5.294	12	0	3	.006	12	-.777	3
477	11	max	95.236	4	382.615	1	-.109	15	0	3	.081	1	.312	1
478		min	3.722	12	-461.775	3	-122.422	1	0	1	-.014	5	-.379	3
479	12	max	87.597	4	299.511	1	1.603	5	0	3	.002	2	.047	1
480		min	3.722	12	-361.538	3	-89.393	1	0	1	-.014	4	-.058	3
481	13	max	79.958	4	216.407	1	3.66	5	0	3	-.004	12	.184	3
482		min	3.722	12	-261.3	3	-56.365	1	0	1	-.058	1	-.154	1
483	14	max	72.319	4	133.303	1	5.716	5	0	3	-.005	12	.348	3
484		min	3.722	12	-161.063	3	-23.336	1	0	1	-.089	1	-.29	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	72.121	1	50.199	1	10.046	4	0	3	-.002	15	.434	3
486		min	3.722	12	-60.826	3	.49	10	0	1	-.094	1	-.361	1
487	16	max	72.121	1	39.412	3	42.721	1	0	3	.004	5	.443	3
488		min	3.722	12	-32.906	1	2.624	12	0	1	-.074	1	-.368	1
489	17	max	72.121	1	139.649	3	75.75	1	0	3	.012	5	.373	3
490		min	3.722	12	-116.01	1	3.914	12	0	1	-.027	1	-.31	1
491	18	max	72.121	1	239.886	3	108.778	1	0	3	.044	1	.226	3
492		min	3.722	12	-199.114	1	5.204	12	0	1	.003	10	-.187	1
493	19	max	72.121	1	340.124	3	141.807	1	0	3	.142	1	0	1
494		min	3.722	12	-282.218	1	6.494	12	0	1	.008	12	0	3
495	M16	1	max	50.617	5	361.533	2	.539	15	0	.139	1	0	2
496		min	-72.447	1	-157.327	3	-141.074	1	0	2	-.028	5	0	3
497	2	max	42.978	5	255.087	2	2.518	5	0	3	.042	1	.104	3
498		min	-72.447	1	-111.135	3	-108.046	1	0	2	-.027	5	-.24	2
499	3	max	35.339	5	148.641	2	4.574	5	0	3	0	12	.173	3
500		min	-72.447	1	-64.944	3	-75.017	1	0	2	-.03	4	-.397	2
501	4	max	27.7	5	42.195	2	6.631	5	0	3	-.003	12	.205	3
502		min	-72.447	1	-18.753	3	-41.988	1	0	2	-.074	1	-.471	2
503	5	max	20.06	5	27.438	3	8.688	5	0	3	-.004	12	.202	3
504		min	-72.447	1	-64.251	2	-8.96	1	0	2	-.094	1	-.462	2
505	6	max	12.421	5	73.629	3	24.069	1	0	3	-.004	15	.163	3
506		min	-72.447	1	-170.697	2	.379	12	0	2	-.088	1	-.371	2
507	7	max	4.782	5	119.82	3	57.097	1	0	3	.003	5	.087	3
508		min	-72.447	1	-277.143	2	1.669	12	0	2	-.057	1	-.197	2
509	8	max	-1.215	12	166.012	3	90.126	1	0	3	.014	4	.06	2
510		min	-72.447	1	-383.589	2	2.959	12	0	2	-.003	3	-.024	3
511	9	max	-1.215	12	212.203	3	123.155	1	0	3	.083	1	.4	2
512		min	-72.447	1	-490.036	2	4.249	12	0	2	0	12	-.171	3
513	10	max	28.217	5	-12.901	15	156.183	1	0	14	.192	1	.822	2
514		min	-73.811	1	-596.482	2	-8.62	3	0	2	.007	12	-.354	3
515	11	max	20.578	5	490.035	2	-.109	15	0	2	.083	1	.4	2
516		min	-73.811	1	-212.203	3	-122.791	1	0	3	-.013	5	-.171	3
517	12	max	12.939	5	383.589	2	1.601	5	0	2	.002	2	.06	2
518		min	-73.811	1	-166.012	3	-89.763	1	0	3	-.013	4	-.024	3
519	13	max	5.3	5	277.143	2	3.658	5	0	2	-.002	12	.087	3
520		min	-73.811	1	-119.82	3	-56.734	1	0	3	-.056	1	-.197	2
521	14	max	-1.453	15	170.697	2	5.714	5	0	2	-.003	12	.163	3
522		min	-73.811	1	-73.629	3	-23.706	1	0	3	-.088	1	-.371	2
523	15	max	-3.973	12	64.251	2	10.019	4	0	2	0	15	.202	3
524		min	-73.811	1	-27.438	3	.498	10	0	3	-.093	1	-.462	2
525	16	max	-3.973	12	18.753	3	42.352	1	0	2	.006	5	.205	3
526		min	-73.811	1	-42.195	2	1.96	12	0	3	-.073	1	-.471	2
527	17	max	-3.973	12	64.944	3	75.38	1	0	2	.014	5	.173	3
528		min	-73.811	1	-148.641	2	3.25	12	0	3	-.027	1	-.397	2
529	18	max	-3.973	12	111.135	3	108.409	1	0	2	.044	1	.104	3
530		min	-73.811	1	-255.087	2	4.54	12	0	3	.003	12	-.24	2
531	19	max	-3.973	12	157.327	3	141.438	1	0	2	.141	1	0	2
532		min	-73.811	1	-361.533	2	5.83	12	0	3	.007	12	0	3
533	M15	1	max	0	2	1.909	1	.043	3	0	1	0	1	1
534		min	-53.43	3	0	2	-.04	1	0	3	0	3	0	1
535	2	max	0	2	1.697	1	.043	3	0	1	0	1	0	2
536		min	-53.501	3	0	2	-.04	1	0	3	0	3	0	1
537	3	max	0	2	1.485	1	.043	3	0	1	0	1	0	2
538		min	-53.571	3	0	2	-.04	1	0	3	0	3	-.001	1
539	4	max	0	2	1.273	1	.043	3	0	1	0	1	0	2
540		min	-53.642	3	0	2	-.04	1	0	3	0	3	-.002	1
541	5	max	0	2	1.06	1	.043	3	0	1	0	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-53.712	3	0	2	-.04	1	0	3	0	3	-.003	1
543		6	max	0	2	.848	1	.043	3	0	1	0	1	0	2
544			min	-53.783	3	0	2	-.04	1	0	3	0	3	-.003	1
545		7	max	0	2	.636	1	.043	3	0	1	0	3	0	2
546			min	-53.853	3	0	2	-.04	1	0	3	0	1	-.003	1
547		8	max	0	2	.424	1	.043	3	0	1	0	3	0	2
548			min	-53.924	3	0	2	-.04	1	0	3	0	1	-.004	1
549		9	max	0	2	.212	1	.043	3	0	1	0	3	0	2
550			min	-53.994	3	0	2	-.04	1	0	3	0	1	-.004	1
551		10	max	0	2	0	1	.043	3	0	1	0	3	0	2
552			min	-54.065	3	0	1	-.04	1	0	3	0	1	-.004	1
553		11	max	0	2	0	2	.043	3	0	1	0	3	0	2
554			min	-54.135	3	-.212	1	-.04	1	0	3	0	1	-.004	1
555		12	max	0	2	0	2	.043	3	0	1	0	3	0	2
556			min	-54.206	3	-.424	1	-.04	1	0	3	0	1	-.004	1
557		13	max	0	2	0	2	.043	3	0	1	0	3	0	2
558			min	-54.276	3	-.636	1	-.04	1	0	3	0	1	-.003	1
559		14	max	0	2	0	2	.043	3	0	1	0	3	0	2
560			min	-54.347	3	-.848	1	-.04	1	0	3	0	1	-.003	1
561		15	max	0	2	0	2	.043	3	0	1	0	3	0	2
562			min	-54.417	3	-1.06	1	-.04	1	0	3	0	1	-.003	1
563		16	max	0	2	0	2	.043	3	0	1	0	3	0	2
564			min	-54.488	3	-1.273	1	-.04	1	0	3	0	1	-.002	1
565		17	max	0	2	0	2	.043	3	0	1	0	3	0	2
566			min	-54.558	3	-1.485	1	-.04	1	0	3	0	1	-.001	1
567		18	max	0	2	0	2	.043	3	0	1	0	3	0	2
568			min	-54.629	3	-1.697	1	-.04	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.043	3	0	1	0	3	0	1
570			min	-54.699	3	-1.909	1	-.04	1	0	3	0	1	0	1
571	M16A	1	max	-.853	10	3.176	4	.284	4	0	3	0	3	0	1
572			min	-236.159	4	.954	12	-.017	3	0	2	0	4	0	1
573		2	max	-.775	10	2.823	4	.256	4	0	3	0	3	0	12
574			min	-236.22	4	.848	12	-.017	3	0	2	0	4	-.001	4
575		3	max	-.697	10	2.47	4	.228	4	0	3	0	3	0	12
576			min	-236.281	4	.742	12	-.017	3	0	2	0	4	-.002	4
577		4	max	-.618	10	2.117	4	.2	4	0	3	0	3	-.001	12
578			min	-236.343	4	.636	12	-.017	3	0	2	0	4	-.003	4
579		5	max	-.54	10	1.764	4	.172	4	0	3	0	3	-.001	12
580			min	-236.404	4	.53	12	-.017	3	0	2	0	1	-.004	4
581		6	max	-.462	10	1.411	4	.143	4	0	3	0	3	-.001	12
582			min	-236.466	4	.424	12	-.017	3	0	2	0	1	-.005	4
583		7	max	-.383	10	1.059	4	.115	4	0	3	0	5	-.002	12
584			min	-236.527	4	.318	12	-.017	3	0	2	0	1	-.005	4
585		8	max	-.305	10	.706	4	.087	4	0	3	0	5	-.002	12
586			min	-236.588	4	.212	12	-.017	3	0	2	0	1	-.006	4
587		9	max	-.227	10	.353	4	.059	4	0	3	0	5	-.002	12
588			min	-236.65	4	.106	12	-.017	3	0	2	0	1	-.006	4
589		10	max	-.148	10	0	1	.031	4	0	3	0	5	-.002	12
590			min	-236.711	4	0	1	-.017	3	0	2	0	1	-.006	4
591		11	max	-.07	10	-.106	12	.023	1	0	3	0	5	-.002	12
592			min	-236.773	4	-.353	4	-.017	3	0	2	0	1	-.006	4
593		12	max	.008	10	-.212	12	.023	1	0	3	0	5	-.002	12
594			min	-236.834	4	-.706	4	-.03	5	0	2	0	1	-.006	4
595		13	max	.087	10	-.318	12	.023	1	0	3	0	5	-.002	12
596			min	-236.895	4	-1.059	4	-.058	5	0	2	0	3	-.005	4
597		14	max	.165	10	-.424	12	.023	1	0	3	0	4	-.001	12
598			min	-236.957	4	-1.411	4	-.086	5	0	2	0	3	-.005	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.243	10	-.53	12	.023	1	0	3	0	4	-.001	12
600		min	-237.018	4	-1.764	4	-.114	5	0	2	0	3	-.004	4
601	16	max	.322	10	-.636	12	.023	1	0	3	0	4	-.001	12
602		min	-237.079	4	-2.117	4	-.142	5	0	2	0	3	-.003	4
603	17	max	.4	10	-.742	12	.023	1	0	3	0	1	0	12
604		min	-237.141	4	-2.47	4	-.171	5	0	2	0	3	-.002	4
605	18	max	.478	10	-.848	12	.023	1	0	3	0	1	0	12
606		min	-237.202	4	-2.823	4	-.199	5	0	2	0	5	-.001	4
607	19	max	.557	10	-.954	12	.023	1	0	3	0	1	0	1
608		min	-237.264	4	-3.176	4	-.227	5	0	2	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.009	2	.014	1	1.767e-3	5	NC	3	NC	3	
2			min	-.004	3	-.009	3	-.017	5	-1.154e-3	1	4253.755	2	2835.966	1	
3		2	max	.003	1	.008	2	.013	1	1.79e-3	5	NC	3	NC	3	
4			min	-.003	3	-.009	3	-.016	5	-1.105e-3	1	4642.982	2	3053.843	1	
5		3	max	.002	1	.008	2	.012	1	1.812e-3	5	NC	3	NC	3	
6			min	-.003	3	-.009	3	-.016	5	-1.055e-3	1	5105.982	2	3311.282	1	
7		4	max	.002	1	.007	2	.011	1	1.835e-3	5	NC	1	NC	3	
8			min	-.003	3	-.008	3	-.015	5	-1.005e-3	1	5660.562	2	3617.714	1	
9		5	max	.002	1	.006	2	.01	1	1.857e-3	5	NC	1	NC	2	
10			min	-.003	3	-.008	3	-.015	5	-9.551e-4	1	6330.454	2	3985.733	1	
11		6	max	.002	1	.006	2	.009	1	1.88e-3	5	NC	1	NC	2	
12			min	-.003	3	-.007	3	-.014	5	-9.053e-4	1	7147.807	2	4432.451	1	
13		7	max	.002	1	.005	2	.008	1	1.903e-3	5	NC	1	NC	2	
14			min	-.002	3	-.007	3	-.013	5	-8.555e-4	1	8157.005	2	4981.599	1	
15		8	max	.002	1	.004	2	.007	1	1.925e-3	5	NC	1	NC	2	
16			min	-.002	3	-.007	3	-.013	5	-8.057e-4	1	9420.683	2	5666.88	1	
17		9	max	.002	1	.004	2	.006	1	1.948e-3	5	NC	1	NC	2	
18			min	-.002	3	-.006	3	-.012	5	-7.558e-4	1	NC	1	6537.5	1	
19		10	max	.001	1	.003	2	.005	1	1.97e-3	5	NC	1	NC	2	
20			min	-.002	3	-.006	3	-.011	5	-7.06e-4	1	NC	1	7667.7	1	
21		11	max	.001	1	.002	2	.004	1	1.993e-3	5	NC	1	NC	2	
22			min	-.002	3	-.005	3	-.01	5	-6.562e-4	1	NC	1	9173.992	1	
23		12	max	.001	1	.002	2	.004	1	2.015e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.009	5	-6.063e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.003	1	2.038e-3	5	NC	1	NC	1	
26			min	-.001	3	-.004	3	-.008	5	-5.565e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	2.061e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.007	5	-5.067e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	2.083e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.006	5	-4.569e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	2.106e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.004	5	-4.07e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.128e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.003	5	-3.572e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.151e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-3.074e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.173e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.576e-4	1	NC	1	NC	1	
39		M3	1	max	0	1	0	1	0	1	1.215e-4	1	NC	1	NC	1
40				min	0	1	0	1	0	1	-1.025e-3	5	NC	1	NC	1
41	2		max	0	3	0	2	.005	5	1.49e-4	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-1.038e-3	5	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.011	5	1.766e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-1.051e-3	5	NC	1	9361.216	14
45		4	max	0	3	0	2	.016	5	2.041e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	-.001	1	-1.065e-3	5	NC	1	6127.279	14
47		5	max	0	3	0	2	.021	5	2.316e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	-1.078e-3	5	NC	1	4523.405	14
49		6	max	0	3	0	2	.026	5	2.591e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.092e-3	5	NC	1	3570.184	14
51		7	max	0	3	0	2	.032	4	2.867e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-1.105e-3	5	NC	1	2941.33	14
53		8	max	0	3	0	2	.037	4	3.142e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.118e-3	5	NC	1	2497.126	14
55		9	max	0	3	.001	2	.042	4	3.417e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-1.132e-3	5	NC	1	2167.799	14
57		10	max	0	3	.002	2	.047	4	3.692e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-1.145e-3	5	NC	1	1914.642	14
59		11	max	.001	3	.002	2	.052	4	3.967e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-1.158e-3	5	NC	1	1714.473	14
61		12	max	.001	3	.003	2	.057	4	4.243e-4	1	NC	1	NC	1
62			min	-.001	2	-.007	3	0	12	-1.172e-3	5	NC	1	1552.567	14
63		13	max	.001	3	.003	2	.062	4	4.518e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	12	-1.185e-3	5	NC	1	1419.128	14
65		14	max	.001	3	.004	2	.067	4	4.793e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	12	-1.198e-3	5	NC	1	1307.386	14
67		15	max	.002	3	.005	2	.072	4	5.068e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	12	-1.212e-3	5	8997.649	2	1212.517	14
69		16	max	.002	3	.006	2	.076	4	5.344e-4	1	NC	1	NC	2
70			min	-.001	2	-.008	3	0	12	-1.225e-3	5	7616.427	2	1130.987	14
71		17	max	.002	3	.007	2	.081	4	5.619e-4	1	NC	1	NC	2
72			min	-.002	2	-.008	3	0	12	-1.238e-3	5	6549.366	2	1060.151	14
73		18	max	.002	3	.008	2	.086	4	5.894e-4	1	NC	3	NC	2
74			min	-.002	2	-.008	3	0	12	-1.252e-3	5	5715.423	2	997.986	14
75		19	max	.002	3	.009	2	.091	4	6.169e-4	1	NC	3	NC	2
76			min	-.002	2	-.008	3	0	12	-1.265e-3	5	5057.729	2	942.92	14
77	M4	1	max	.002	1	.011	2	0	12	6.353e-3	5	NC	1	NC	3
78			min	0	12	-.009	3	-.096	4	-8.995e-4	1	NC	1	202.015	4
79		2	max	.002	1	.01	2	0	12	6.353e-3	5	NC	1	NC	3
80			min	0	12	-.009	3	-.088	4	-8.995e-4	1	NC	1	220.224	4
81		3	max	.002	1	.01	2	0	12	6.353e-3	5	NC	1	NC	2
82			min	0	12	-.008	3	-.08	4	-8.995e-4	1	NC	1	241.896	4
83		4	max	.002	1	.009	2	0	12	6.353e-3	5	NC	1	NC	2
84			min	0	12	-.008	3	-.072	4	-8.995e-4	1	NC	1	267.946	4
85		5	max	.002	1	.008	2	0	12	6.353e-3	5	NC	1	NC	2
86			min	0	12	-.007	3	-.065	4	-8.995e-4	1	NC	1	299.615	4
87		6	max	.001	1	.008	2	0	12	6.353e-3	5	NC	1	NC	2
88			min	0	12	-.007	3	-.057	4	-8.995e-4	1	NC	1	338.632	4
89		7	max	.001	1	.007	2	0	12	6.353e-3	5	NC	1	NC	2
90			min	0	12	-.006	3	-.05	4	-8.995e-4	1	NC	1	387.46	4
91		8	max	.001	1	.007	2	0	12	6.353e-3	5	NC	1	NC	2
92			min	0	12	-.006	3	-.043	4	-8.995e-4	1	NC	1	449.703	4
93		9	max	.001	1	.006	2	0	12	6.353e-3	5	NC	1	NC	2
94			min	0	12	-.005	3	-.036	4	-8.995e-4	1	NC	1	530.815	4
95		10	max	.001	1	.005	2	0	12	6.353e-3	5	NC	1	NC	1
96			min	0	12	-.005	3	-.03	4	-8.995e-4	1	NC	1	639.365	4
97		11	max	0	1	.005	2	0	12	6.353e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.024	4	-8.995e-4	1	NC	1	789.473	4
99		12	max	0	1	.004	2	0	12	6.353e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	12	-.004	3	-.019	4	-8.995e-4	1	NC	1	1005.762	4
101		max	0	1	.004	2	0	12	6.353e-3	5	NC	1	NC	1
102		min	0	12	-.003	3	-.014	4	-8.995e-4	1	NC	1	1334.422	4
103		max	0	1	.003	2	0	12	6.353e-3	5	NC	1	NC	1
104		min	0	12	-.003	3	-.01	4	-8.995e-4	1	NC	1	1870.731	4
105		max	0	1	.002	2	0	12	6.353e-3	5	NC	1	NC	1
106		min	0	12	-.002	3	-.007	4	-8.995e-4	1	NC	1	2838.526	4
107		max	0	1	.002	2	0	12	6.353e-3	5	NC	1	NC	1
108		min	0	12	-.002	3	-.004	4	-8.995e-4	1	NC	1	4874.137	4
109		max	0	1	.001	2	0	12	6.353e-3	5	NC	1	NC	1
110		min	0	12	-.001	3	-.002	4	-8.995e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	12	6.353e-3	5	NC	1	NC	1
112		min	0	12	0	3	0	4	-8.995e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	6.353e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-8.995e-4	1	NC	1	NC	1
115	M6	max	.009	1	.034	2	.005	1	1.937e-3	4	NC	3	NC	2
116		min	-.011	3	-.03	3	-.017	5	9.103e-7	10	1174.557	2	8317.329	1
117		max	.009	1	.031	2	.004	1	1.956e-3	4	NC	3	NC	2
118		min	-.011	3	-.028	3	-.016	5	2.811e-7	10	1256.107	2	9005.318	1
119		max	.008	1	.029	2	.004	1	1.975e-3	4	NC	3	NC	2
120		min	-.01	3	-.027	3	-.016	5	-3.481e-7	10	1349.457	2	9818.967	1
121		max	.008	1	.027	2	.004	1	1.993e-3	4	NC	3	NC	1
122		min	-.01	3	-.025	3	-.015	5	-1.145e-6	2	1456.964	2	NC	1
123		max	.007	1	.025	2	.003	1	2.012e-3	4	NC	3	NC	1
124		min	-.009	3	-.023	3	-.015	5	-3.903e-6	2	1581.663	2	NC	1
125		max	.007	1	.023	2	.003	1	2.031e-3	4	NC	3	NC	1
126		min	-.008	3	-.022	3	-.014	5	-6.661e-6	2	1727.526	2	NC	1
127		max	.006	1	.021	2	.003	1	2.049e-3	4	NC	3	NC	1
128		min	-.008	3	-.02	3	-.014	5	-9.418e-6	2	1899.852	2	NC	1
129		max	.006	1	.019	2	.002	1	2.068e-3	4	NC	3	NC	1
130		min	-.007	3	-.019	3	-.013	5	-1.218e-5	2	2105.867	2	NC	1
131		max	.005	1	.017	2	.002	1	2.087e-3	4	NC	3	NC	1
132		min	-.006	3	-.017	3	-.012	5	-1.493e-5	2	2355.691	2	NC	1
133		max	.005	1	.015	2	.002	1	2.106e-3	4	NC	3	NC	1
134		min	-.006	3	-.015	3	-.011	5	-1.769e-5	2	2663.937	2	NC	1
135		max	.004	1	.013	2	.001	1	2.124e-3	4	NC	3	NC	1
136		min	-.005	3	-.014	3	-.011	5	-2.045e-5	2	3052.517	2	NC	1
137		max	.004	1	.011	2	.001	1	2.143e-3	4	NC	3	NC	1
138		min	-.004	3	-.012	3	-.009	5	-2.321e-5	2	3555.858	2	NC	1
139		max	.003	1	.009	2	0	1	2.162e-3	4	NC	3	NC	1
140		min	-.004	3	-.01	3	-.008	5	-2.596e-5	2	4231.319	2	NC	1
141		max	.003	1	.008	2	0	1	2.18e-3	4	NC	3	NC	1
142		min	-.003	3	-.009	3	-.007	5	-2.872e-5	2	5182.118	2	NC	1
143		max	.002	1	.006	2	0	1	2.199e-3	4	NC	3	NC	1
144		min	-.003	3	-.007	3	-.006	5	-3.148e-5	2	6614.65	2	NC	1
145		max	.002	1	.004	2	0	1	2.218e-3	4	NC	1	NC	1
146		min	-.002	3	-.005	3	-.004	5	-3.424e-5	2	9010.432	2	NC	1
147		max	.001	1	.003	2	0	1	2.237e-3	4	NC	1	NC	1
148		min	-.001	3	-.003	3	-.003	5	-3.699e-5	2	NC	1	NC	1
149		max	0	1	.001	2	0	1	2.255e-3	4	NC	1	NC	1
150		min	0	3	-.002	3	-.002	5	-4.63e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	2.276e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-5.736e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.663e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-1.073e-3	5	NC	1	NC	1
155		max	0	3	.002	2	.005	5	2.405e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-1.072e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.011	5	2.147e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-1.071e-3	4	NC	1	NC	1
159		4	max	.001	3	.005	2	.016	5	1.889e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.071e-3	4	NC	1	NC	1
161		5	max	.001	3	.006	2	.022	5	1.631e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-1.071e-3	4	7690.039	2	NC	1
163		6	max	.002	3	.007	2	.027	5	2.105e-5	3	NC	3	NC	1
164			min	-.002	2	-.01	3	0	1	-1.07e-3	4	6165.667	2	NC	1
165		7	max	.002	3	.009	2	.033	4	3.807e-5	3	NC	3	NC	1
166			min	-.002	2	-.012	3	0	1	-1.07e-3	4	5125.995	2	NC	1
167		8	max	.002	3	.011	2	.038	4	5.508e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-1.07e-3	4	4365.738	2	NC	1
169		9	max	.003	3	.012	2	.043	4	7.21e-5	3	NC	3	NC	1
170			min	-.003	2	-.015	3	-.001	1	-1.069e-3	4	3782.554	2	NC	1
171		10	max	.003	3	.014	2	.048	4	8.911e-5	3	NC	3	NC	1
172			min	-.004	2	-.016	3	-.001	1	-1.069e-3	4	3319.706	2	NC	1
173		11	max	.003	3	.016	2	.053	4	1.061e-4	3	NC	3	NC	1
174			min	-.004	2	-.018	3	-.001	1	-1.069e-3	4	2943.145	2	NC	1
175		12	max	.004	3	.018	2	.058	4	1.231e-4	3	NC	3	NC	1
176			min	-.004	2	-.019	3	-.002	1	-1.068e-3	4	2631.116	2	NC	1
177		13	max	.004	3	.019	2	.063	4	1.402e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	-.002	1	-1.068e-3	4	2368.987	2	NC	1
179		14	max	.004	3	.021	2	.068	4	1.572e-4	3	NC	3	NC	1
180			min	-.005	2	-.022	3	-.002	1	-1.067e-3	4	2146.483	2	NC	1
181		15	max	.005	3	.024	2	.073	4	1.742e-4	3	NC	3	NC	1
182			min	-.006	2	-.023	3	-.002	1	-1.067e-3	4	1956.123	2	NC	1
183		16	max	.005	3	.026	2	.077	4	1.912e-4	3	NC	3	NC	1
184			min	-.006	2	-.024	3	-.002	1	-1.067e-3	4	1792.293	2	NC	1
185		17	max	.005	3	.028	2	.082	4	2.082e-4	3	NC	3	NC	1
186			min	-.006	2	-.025	3	-.002	1	-1.066e-3	4	1650.671	2	NC	1
187		18	max	.006	3	.03	2	.086	4	2.252e-4	3	NC	3	NC	1
188			min	-.007	2	-.026	3	-.002	1	-1.066e-3	4	1527.868	2	NC	1
189		19	max	.006	3	.032	2	.091	4	2.422e-4	3	NC	3	NC	1
190			min	-.007	2	-.026	3	-.002	1	-1.066e-3	4	1421.179	2	NC	1
191	M8	1	max	.005	1	.038	2	.002	1	6.169e-3	4	NC	1	NC	2
192			min	0	3	-.029	3	-.096	4	-1.914e-4	3	NC	1	201.701	4
193		2	max	.005	1	.036	2	.002	1	6.169e-3	4	NC	1	NC	2
194			min	0	3	-.028	3	-.088	4	-1.914e-4	3	NC	1	219.88	4
195		3	max	.005	1	.034	2	.002	1	6.169e-3	4	NC	1	NC	2
196			min	0	3	-.026	3	-.08	4	-1.914e-4	3	NC	1	241.518	4
197		4	max	.004	1	.032	2	.002	1	6.169e-3	4	NC	1	NC	1
198			min	0	3	-.025	3	-.072	4	-1.914e-4	3	NC	1	267.526	4
199		5	max	.004	1	.03	2	.002	1	6.169e-3	4	NC	1	NC	1
200			min	0	3	-.023	3	-.065	4	-1.914e-4	3	NC	1	299.144	4
201		6	max	.004	1	.028	2	.001	1	6.169e-3	4	NC	1	NC	1
202			min	0	3	-.021	3	-.057	4	-1.914e-4	3	NC	1	338.099	4
203		7	max	.004	1	.026	2	.001	1	6.169e-3	4	NC	1	NC	1
204			min	0	3	-.02	3	-.05	4	-1.914e-4	3	NC	1	386.848	4
205		8	max	.003	1	.023	2	.001	1	6.169e-3	4	NC	1	NC	1
206			min	0	3	-.018	3	-.043	4	-1.914e-4	3	NC	1	448.992	4
207		9	max	.003	1	.021	2	0	1	6.169e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.036	4	-1.914e-4	3	NC	1	529.973	4
209		10	max	.003	1	.019	2	0	1	6.169e-3	4	NC	1	NC	1
210			min	0	3	-.015	3	-.03	4	-1.914e-4	3	NC	1	638.349	4
211		11	max	.002	1	.017	2	0	1	6.169e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.025	4	-1.914e-4	3	NC	1	788.216	4
213		12	max	.002	1	.015	2	0	1	6.169e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.009	4	2.779e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.037e-3	4	NC	1	5378.504	5
273		4	max	0	3	0	2	.013	4	9.446e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.135e-3	4	NC	1	3559.415	5
275		5	max	0	3	0	2	.017	4	-6.331e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-1.232e-3	4	NC	1	2654.115	5
277		6	max	0	3	0	2	.022	5	-1.755e-5	12	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-1.329e-3	4	NC	1	2113.864	5
279		7	max	0	3	0	2	.026	5	-2.877e-5	12	NC	1	NC	1
280			min	0	2	-.005	3	-.002	1	-1.427e-3	4	NC	1	1755.784	5
281		8	max	0	3	0	2	.031	5	-3.432e-5	10	NC	1	NC	1
282			min	0	2	-.006	3	-.003	1	-1.524e-3	4	NC	1	1501.519	5
283		9	max	0	3	.001	2	.035	5	-3.832e-5	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	1	-1.621e-3	4	NC	1	1311.903	5
285		10	max	0	3	.002	2	.04	5	-4.232e-5	10	NC	1	NC	1
286			min	0	2	-.007	3	-.004	1	-1.718e-3	4	NC	1	1165.187	5
287		11	max	.001	3	.002	2	.044	5	-4.632e-5	10	NC	1	NC	2
288			min	0	2	-.007	3	-.005	1	-1.816e-3	4	NC	1	1048.324	5
289		12	max	.001	3	.003	2	.048	5	-5.031e-5	10	NC	1	NC	2
290			min	-.001	2	-.008	3	-.006	1	-1.913e-3	4	NC	1	953.015	5
291		13	max	.001	3	.003	2	.053	5	-5.431e-5	10	NC	1	NC	2
292			min	-.001	2	-.008	3	-.007	1	-2.01e-3	4	NC	1	873.726	5
293		14	max	.001	3	.004	2	.057	5	-5.831e-5	10	NC	1	NC	2
294			min	-.001	2	-.008	3	-.009	1	-2.107e-3	4	NC	1	806.629	5
295		15	max	.002	3	.005	2	.062	5	-6.23e-5	10	NC	1	NC	2
296			min	-.001	2	-.008	3	-.01	1	-2.205e-3	4	9009.643	2	748.987	5
297		16	max	.002	3	.006	2	.066	5	-6.63e-5	10	NC	1	NC	2
298			min	-.001	2	-.008	3	-.011	1	-2.302e-3	4	7625.671	2	698.794	5
299		17	max	.002	3	.007	2	.07	5	-7.03e-5	10	NC	1	NC	2
300			min	-.002	2	-.008	3	-.012	1	-2.399e-3	4	6556.682	2	654.546	5
301		18	max	.002	3	.008	2	.075	5	-7.43e-5	10	NC	3	NC	3
302			min	-.002	2	-.008	3	-.013	1	-2.497e-3	4	5721.362	2	615.094	5
303		19	max	.002	3	.009	2	.079	5	-7.829e-5	10	NC	3	NC	3
304			min	-.002	2	-.008	3	-.014	1	-2.594e-3	4	5062.669	2	579.548	5
305	M12	1	max	.002	1	.011	2	.012	1	7.561e-3	4	NC	1	NC	3
306			min	0	12	-.009	3	-.088	5	8.072e-5	10	NC	1	220.316	5
307		2	max	.002	1	.01	2	.011	1	7.561e-3	4	NC	1	NC	3
308			min	0	12	-.009	3	-.08	5	8.072e-5	10	NC	1	240.168	5
309		3	max	.002	1	.01	2	.01	1	7.561e-3	4	NC	1	NC	3
310			min	0	12	-.008	3	-.073	5	8.072e-5	10	NC	1	263.797	5
311		4	max	.002	1	.009	2	.009	1	7.561e-3	4	NC	1	NC	3
312			min	0	12	-.008	3	-.066	5	8.072e-5	10	NC	1	292.198	5
313		5	max	.002	1	.008	2	.008	1	7.561e-3	4	NC	1	NC	3
314			min	0	12	-.007	3	-.059	5	8.072e-5	10	NC	1	326.725	5
315		6	max	.001	1	.008	2	.007	1	7.561e-3	4	NC	1	NC	3
316			min	0	12	-.007	3	-.052	5	8.072e-5	10	NC	1	369.263	5
317		7	max	.001	1	.007	2	.006	1	7.561e-3	4	NC	1	NC	3
318			min	0	12	-.006	3	-.046	5	8.072e-5	10	NC	1	422.496	5
319		8	max	.001	1	.007	2	.005	1	7.561e-3	4	NC	1	NC	2
320			min	0	12	-.006	3	-.039	5	8.072e-5	10	NC	1	490.354	5
321		9	max	.001	1	.006	2	.004	1	7.561e-3	4	NC	1	NC	2
322			min	0	12	-.005	3	-.033	5	8.072e-5	10	NC	1	578.78	5
323		10	max	.001	1	.005	2	.004	1	7.561e-3	4	NC	1	NC	2
324			min	0	12	-.005	3	-.028	5	8.072e-5	10	NC	1	697.12	5
325		11	max	0	1	.005	2	.003	1	7.561e-3	4	NC	1	NC	2
326			min	0	12	-.004	3	-.022	5	8.072e-5	10	NC	1	860.761	5
327		12	max	0	1	.004	2	.002	1	7.561e-3	4	NC	1	NC	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	12	-.004	3	-.018	5	8.072e-5	10	NC	1	1096.547	5
329		max	0	1	.004	2	.002	1	7.561e-3	4	NC	1	NC	1
330		min	0	12	-.003	3	-.013	5	8.072e-5	10	NC	1	1454.829	5
331		max	0	1	.003	2	.001	1	7.561e-3	4	NC	1	NC	1
332		min	0	12	-.003	3	-.009	5	8.072e-5	10	NC	1	2039.464	5
333		max	0	1	.002	2	0	1	7.561e-3	4	NC	1	NC	1
334		min	0	12	-.002	3	-.006	5	8.072e-5	10	NC	1	3094.452	5
335		max	0	1	.002	2	0	1	7.561e-3	4	NC	1	NC	1
336		min	0	12	-.002	3	-.004	5	8.072e-5	10	NC	1	5313.419	5
337		max	0	1	.001	2	0	1	7.561e-3	4	NC	1	NC	1
338		min	0	12	-.001	3	-.002	5	8.072e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	7.561e-3	4	NC	1	NC	1
340		min	0	12	0	3	0	5	8.072e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	7.561e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	8.072e-5	10	NC	1	NC	1
343	M1	max	.008	3	.025	3	.009	5	1.903e-2	1	NC	1	NC	1
344		min	-.009	2	-.023	2	-.005	1	-2.279e-2	3	NC	1	NC	1
345		max	.008	3	.015	3	.012	5	9.078e-3	1	NC	4	NC	2
346		min	-.009	2	-.013	2	-.01	1	-1.128e-2	3	4709.547	2	8466.37	1
347		max	.008	3	.005	3	.017	5	5.916e-4	5	NC	4	NC	2
348		min	-.009	2	-.004	2	-.014	1	-6.847e-4	1	2417.243	2	5136.82	1
349		max	.008	3	.004	1	.021	5	6.051e-4	5	NC	4	NC	2
350		min	-.009	2	-.003	3	-.016	1	-5.867e-4	1	1690.958	2	3756.485	5
351		max	.008	3	.011	2	.026	5	6.185e-4	5	NC	5	NC	2
352		min	-.009	2	-.009	3	-.016	1	-4.887e-4	1	1339.529	2	2692.961	5
353		max	.008	3	.017	2	.031	5	6.32e-4	5	NC	5	NC	2
354		min	-.009	2	-.014	3	-.015	1	-3.906e-4	1	1138.8	2	2071.657	5
355		max	.008	3	.021	2	.037	5	6.454e-4	5	NC	5	NC	2
356		min	-.009	2	-.018	3	-.013	1	-2.926e-4	1	1015.062	2	1668.973	5
357		max	.008	3	.025	2	.042	5	6.589e-4	5	NC	5	NC	2
358		min	-.009	2	-.021	3	-.011	1	-1.946e-4	1	937.518	2	1389.611	5
359		max	.008	3	.027	2	.048	5	6.724e-4	5	NC	5	NC	1
360		min	-.009	2	-.023	3	-.008	1	-9.659e-5	1	891.628	2	1183.478	4
361		max	.008	3	.028	2	.054	5	6.858e-4	5	NC	5	NC	1
362		min	-.009	2	-.023	3	-.004	1	-1.545e-6	11	870.461	2	1012.602	4
363		max	.008	3	.027	2	.06	4	7.217e-4	4	NC	5	NC	1
364		min	-.009	2	-.022	3	-.001	1	1.738e-5	10	871.523	2	884.235	4
365		max	.008	3	.025	2	.067	4	7.578e-4	4	NC	5	NC	2
366		min	-.009	2	-.021	3	0	10	2.656e-5	10	895.83	2	785.617	4
367		max	.008	3	.022	2	.073	4	7.938e-4	4	NC	5	NC	2
368		min	-.009	2	-.018	3	0	12	3.173e-5	12	948.452	2	708.574	4
369		max	.008	3	.017	2	.08	4	8.299e-4	4	NC	5	NC	2
370		min	-.009	2	-.014	3	0	12	3.497e-5	12	1041.086	2	647.679	4
371		max	.008	3	.011	2	.085	4	8.66e-4	4	NC	4	NC	2
372		min	-.009	2	-.009	3	0	12	3.822e-5	12	1199.397	2	599.222	4
373		max	.008	3	.003	2	.091	4	1.227e-3	4	NC	4	NC	2
374		min	-.009	2	-.003	3	0	12	4.048e-5	12	1486.13	2	560.612	4
375		max	.008	3	.004	3	.096	4	9.321e-3	4	NC	4	NC	2
376		min	-.009	2	-.006	2	0	12	-7.703e-5	1	2101.78	2	530.044	4
377		max	.008	3	.012	3	.1	4	1.207e-2	2	NC	4	NC	2
378		min	-.009	2	-.018	2	0	10	-5.355e-3	3	4070.938	2	506.16	4
379		max	.008	3	.02	3	.103	4	2.44e-2	2	NC	1	NC	1
380		min	-.009	2	-.029	2	-.003	1	-1.084e-2	3	NC	1	488.599	4
381	M5	max	.027	3	.081	3	.008	5	8.672e-6	4	NC	1	NC	1
382		min	-.031	2	-.077	2	-.006	1	4.783e-8	10	NC	1	NC	1
383		max	.027	3	.048	3	.012	5	2.991e-4	5	NC	5	NC	1
384		min	-.031	2	-.044	2	-.005	1	-6.538e-5	1	1402.362	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.027	3	.017	3	.017	5	5.846e-4	5	NC	5	NC	1
386			min	-.031	2	-.014	2	-.005	1	-1.302e-4	1	719.344	2	NC	1
387		4	max	.026	3	.013	2	.022	5	6.088e-4	5	NC	5	NC	1
388			min	-.031	2	-.009	3	-.004	1	-1.239e-4	1	502.67	2	NC	1
389		5	max	.026	3	.036	2	.027	5	6.33e-4	5	NC	5	NC	1
390			min	-.031	2	-.03	3	-.004	1	-1.177e-4	1	397.777	2	NC	1
391		6	max	.026	3	.056	2	.033	5	6.571e-4	5	NC	15	NC	1
392			min	-.031	2	-.047	3	-.004	1	-1.114e-4	1	337.828	2	NC	1
393		7	max	.026	3	.071	2	.039	5	6.813e-4	5	NC	15	NC	1
394			min	-.031	2	-.06	3	-.003	1	-1.052e-4	1	300.836	2	NC	1
395		8	max	.026	3	.083	2	.045	5	7.055e-4	5	NC	15	NC	1
396			min	-.031	2	-.069	3	-.003	1	-9.891e-5	1	277.613	2	NC	1
397		9	max	.026	3	.091	2	.051	5	7.296e-4	5	NC	15	NC	1
398			min	-.031	2	-.074	3	-.003	1	-9.266e-5	1	263.816	2	NC	1
399		10	max	.026	3	.094	2	.057	5	7.538e-4	5	NC	15	NC	1
400			min	-.031	2	-.076	3	-.003	1	-8.641e-5	1	257.371	2	NC	1
401		11	max	.026	3	.092	2	.063	5	7.78e-4	5	NC	15	NC	1
402			min	-.031	2	-.073	3	-.003	1	-8.016e-5	1	257.526	2	NC	1
403		12	max	.026	3	.086	2	.069	5	8.021e-4	5	NC	15	NC	1
404			min	-.031	2	-.067	3	-.002	1	-7.391e-5	1	264.573	2	NC	1
405		13	max	.026	3	.075	2	.075	5	8.263e-4	5	NC	15	NC	1
406			min	-.031	2	-.057	3	-.002	1	-6.766e-5	1	280.006	2	NC	1
407		14	max	.026	3	.059	2	.081	4	8.504e-4	5	NC	15	NC	1
408			min	-.031	2	-.044	3	-.002	1	-6.141e-5	1	307.284	2	NC	1
409		15	max	.026	3	.037	2	.086	4	8.746e-4	5	NC	5	NC	1
410			min	-.031	2	-.028	3	-.002	1	-5.516e-5	1	354.018	2	NC	1
411		16	max	.025	3	.011	2	.091	4	1.222e-3	5	NC	5	NC	1
412			min	-.031	2	-.008	3	-.002	1	-5.587e-5	1	438.866	2	NC	1
413		17	max	.025	3	.014	3	.096	4	9.303e-3	4	NC	5	NC	1
414			min	-.031	2	-.022	2	-.002	1	-2.222e-4	1	621.959	2	NC	1
415		18	max	.025	3	.039	3	.1	4	4.772e-3	4	NC	5	NC	1
416			min	-.031	2	-.06	2	-.002	1	-1.137e-4	1	1205.82	2	NC	1
417		19	max	.026	3	.065	3	.103	4	2.376e-6	5	NC	1	NC	1
418			min	-.031	2	-.1	2	-.003	1	-3.158e-7	3	NC	1	NC	1
419	M9	1	max	.008	3	.024	3	.007	5	2.279e-2	3	NC	1	NC	1
420			min	-.009	2	-.023	2	-.007	1	-1.902e-2	1	NC	1	NC	1
421		2	max	.008	3	.014	3	.007	5	1.127e-2	3	NC	4	NC	2
422			min	-.009	2	-.013	2	-.001	1	-9.307e-3	1	4711.755	2	9488.214	1
423		3	max	.008	3	.005	3	.007	4	2.3e-4	1	NC	4	NC	2
424			min	-.009	2	-.004	2	0	3	-3.455e-5	3	2418.406	2	5857.704	1
425		4	max	.008	3	.004	2	.009	4	1.472e-4	1	NC	4	NC	2
426			min	-.009	2	-.003	3	0	3	-4.131e-5	3	1691.786	2	4935.42	1
427		5	max	.008	3	.011	2	.012	4	6.443e-5	1	NC	4	NC	2
428			min	-.009	2	-.009	3	-.002	3	-4.808e-5	3	1340.183	2	4854.015	1
429		6	max	.008	3	.017	2	.015	4	4.47e-5	4	NC	5	NC	2
430			min	-.009	2	-.015	3	-.002	3	-5.484e-5	3	1139.349	2	4611.026	4
431		7	max	.008	3	.021	2	.019	4	6.01e-5	5	NC	5	NC	2
432			min	-.009	2	-.019	3	-.003	3	-1.011e-4	1	1015.543	2	3305.009	4
433		8	max	.008	3	.025	2	.024	4	7.665e-5	5	NC	5	NC	1
434			min	-.009	2	-.021	3	-.003	3	-1.839e-4	1	937.954	2	2493.266	4
435		9	max	.008	3	.027	2	.029	5	9.319e-5	5	NC	5	NC	1
436			min	-.009	2	-.023	3	-.003	3	-2.667e-4	1	892.034	2	1954.902	4
437		10	max	.008	3	.028	2	.035	5	1.097e-4	5	NC	5	NC	1
438			min	-.009	2	-.023	3	-.006	1	-3.494e-4	1	870.849	2	1579.564	4
439		11	max	.008	3	.027	2	.042	5	1.263e-4	5	NC	5	NC	1
440			min	-.009	2	-.023	3	-.009	1	-4.322e-4	1	871.903	2	1307.353	4
441		12	max	.008	3	.025	2	.049	5	1.428e-4	5	NC	5	NC	2







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.198	3	.098	1	6.886e-3	2	NC	5	NC	3
500		min	-.103	4	-.429	2	.005	10	-4.567e-3	3	420.021	2	1631.332	1
501	4	max	.002	1	.25	3	.148	1	8.078e-3	2	NC	15	NC	10
502		min	-.103	4	-.543	2	.009	10	-5.324e-3	3	327.35	2	1099.053	1
503	5	max	.002	1	.266	3	.172	1	9.27e-3	2	NC	15	NC	10
504		min	-.103	4	-.576	2	.01	10	-6.082e-3	3	307.173	2	949.887	1
505	6	max	.002	1	.249	3	.163	1	1.046e-2	2	NC	15	NC	10
506		min	-.103	4	-.532	2	.007	10	-6.839e-3	3	334.274	2	1000.593	1
507	7	max	.002	1	.204	3	.123	1	1.165e-2	2	NC	5	NC	5
508		min	-.103	4	-.425	2	.001	10	-7.597e-3	3	424.411	2	1311.732	1
509	8	max	.002	1	.145	3	.064	1	1.285e-2	2	NC	5	NC	2
510		min	-.103	4	-.286	2	-.007	10	-8.354e-3	3	654.841	2	2445.739	1
511	9	max	.002	1	.09	3	.028	3	1.404e-2	2	NC	5	NC	1
512		min	-.103	4	-.158	2	-.02	2	-9.111e-3	3	1305	2	8348.697	3
513	10	max	.003	1	.065	3	.026	3	1.523e-2	2	NC	4	NC	4
514		min	-.103	4	-.1	2	-.031	2	-9.869e-3	3	2379.131	2	7472.435	2
515	11	max	.003	1	.09	3	.025	3	1.404e-2	2	NC	5	NC	1
516		min	-.103	4	-.158	2	-.019	2	-9.111e-3	3	1305	2	9831.738	3
517	12	max	.003	1	.145	3	.063	1	1.285e-2	2	NC	5	NC	2
518		min	-.103	4	-.286	2	-.007	10	-8.352e-3	3	654.841	2	2490.285	1
519	13	max	.003	1	.204	3	.122	1	1.165e-2	2	NC	5	NC	5
520		min	-.103	4	-.425	2	.001	10	-7.594e-3	3	424.411	2	1331.31	1
521	14	max	.003	1	.249	3	.161	1	1.046e-2	2	NC	15	NC	5
522		min	-.103	4	-.532	2	.005	15	-6.836e-3	3	334.274	2	1015.287	1
523	15	max	.003	1	.266	3	.17	1	9.271e-3	2	NC	15	NC	3
524		min	-.103	4	-.576	2	0	15	-6.077e-3	3	307.173	2	964.986	1
525	16	max	.003	1	.25	3	.145	1	8.08e-3	2	NC	15	NC	3
526		min	-.103	4	-.543	2	-.005	5	-5.319e-3	3	327.35	2	1119.516	1
527	17	max	.003	1	.198	3	.096	1	6.888e-3	2	NC	5	NC	3
528		min	-.103	4	-.429	2	-.009	5	-4.561e-3	3	420.021	2	1670.411	1
529	18	max	.003	1	.118	3	.037	1	5.697e-3	2	NC	5	NC	2
530		min	-.103	4	-.249	2	-.009	5	-3.802e-3	3	763.842	2	3998.072	1
531	19	max	.003	1	.02	3	.008	3	4.505e-3	2	NC	1	NC	1
532		min	-.103	4	-.029	2	-.009	2	-3.044e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.746e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.688e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.011	4	8.88e-4	3	NC	3	NC	1
536		min	0	5	-.012	1	0	3	-6.971e-4	5	7510.717	1	8440.518	4
537	3	max	0	3	0	15	.024	4	1.401e-3	3	NC	5	NC	1
538		min	-.002	5	-.024	1	-.004	3	-1.157e-3	2	3821.948	1	3837.845	4
539	4	max	0	3	0	15	.038	4	1.915e-3	3	NC	5	NC	9
540		min	-.003	5	-.036	1	-.007	3	-1.703e-3	2	2622.079	1	2443.062	4
541	5	max	0	3	-.001	15	.051	4	2.428e-3	3	NC	5	NC	9
542		min	-.003	5	-.046	1	-.012	3	-2.25e-3	2	2046.035	1	1816.675	4
543	6	max	0	3	-.001	15	.062	4	2.942e-3	3	NC	5	9114.605	9
544		min	-.004	5	-.054	1	-.018	3	-2.796e-3	2	1721.955	1	1487.421	4
545	7	max	0	3	-.001	15	.071	4	3.455e-3	3	NC	5	7199.055	9
546		min	-.005	5	-.061	1	-.023	3	-3.342e-3	2	1527.063	1	1305.741	4
547	8	max	0	3	-.001	15	.076	4	3.969e-3	3	NC	5	5983.081	9
548		min	-.006	5	-.066	1	-.029	3	-3.888e-3	2	1410.099	1	1212.552	4
549	9	max	0	3	-.001	15	.078	4	4.482e-3	3	NC	5	5182.463	9
550		min	-.007	5	-.07	1	-.034	3	-4.435e-3	2	1347.142	1	1183.239	4
551	10	max	0	3	0	15	.076	4	4.996e-3	3	NC	5	4652.886	9
552		min	-.008	5	-.071	1	-.038	3	-4.981e-3	2	1327.225	1	1210.595	4
553	11	max	0	3	0	15	.071	4	5.509e-3	3	NC	5	4318.244	9
554		min	-.009	5	-.07	1	-.04	3	-5.527e-3	2	1347.142	1	1300.473	4
555	12	max	0	3	0	15	.062	4	6.022e-3	3	NC	5	4141.197	9



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.009	5	-0.067	1	-0.041	3	-6.073e-3	2	1410.099	1	1474.823	4
557		max	0	3	0	15	.051	4	6.536e-3	3	NC	5	4112.289	9
558		min	-.01	5	-.062	1	-.04	3	-6.619e-3	2	1527.063	1	1515.593	3
559		max	0	3	0	15	.039	4	7.049e-3	3	NC	5	4369.632	15
560		min	-.011	5	-.055	1	-.036	3	-7.166e-3	2	1721.955	1	1563.26	3
561		max	.001	3	.001	15	.03	1	7.563e-3	3	NC	5	7185.52	15
562		min	-.012	5	-.047	1	-.03	3	-7.712e-3	2	2046.035	1	1697.651	3
563		max	.001	3	.002	5	.021	1	8.076e-3	3	NC	5	NC	5
564		min	-.013	5	-.037	1	-.02	3	-8.258e-3	2	2622.079	1	1984.817	3
565		max	.001	3	.004	5	.009	1	8.59e-3	3	NC	5	NC	4
566		min	-.014	5	-.026	1	-.007	3	-8.804e-3	2	3821.948	1	2631.916	3
567		max	.001	3	.005	5	.01	3	9.103e-3	3	NC	3	NC	4
568		min	-.014	5	-.014	1	-.014	2	-9.351e-3	2	7510.717	1	4686.817	3
569		max	.001	3	.007	5	.032	3	9.617e-3	3	NC	1	NC	1
570		min	-.015	5	-.003	9	-.034	2	-9.897e-3	2	NC	1	NC	1
571	M16A	max	0	10	0	2	.01	3	2.841e-3	3	NC	1	NC	1
572		min	-.006	4	-.004	4	-.01	2	-2.753e-3	2	NC	1	NC	1
573		max	0	10	-.006	12	.004	1	2.727e-3	3	NC	3	NC	2
574		min	-.005	4	-.024	4	-.004	5	-2.632e-3	2	4514.793	4	9830.363	1
575		max	0	10	-.012	12	.011	1	2.612e-3	3	7643.895	12	NC	4
576		min	-.005	4	-.044	4	-.012	5	-2.512e-3	2	2297.424	4	5557.605	1
577		max	0	10	-.018	12	.017	1	2.497e-3	3	5244.159	12	NC	10
578		min	-.005	4	-.062	4	-.024	5	-2.391e-3	2	1576.167	4	4190.307	5
579		max	0	10	-.023	12	.021	1	2.383e-3	3	4092.07	12	NC	10
580		min	-.005	4	-.079	4	-.037	5	-2.27e-3	2	1229.899	4	2576.626	5
581		max	0	10	-.027	12	.023	1	2.268e-3	3	3443.91	12	9846.981	10
582		min	-.004	4	-.093	4	-.052	5	-2.15e-3	2	1035.09	4	1845.191	5
583		max	0	10	-.03	12	.024	1	2.153e-3	3	3054.126	12	9712.976	10
584		min	-.004	4	-.104	4	-.065	5	-2.029e-3	2	917.938	4	1458.153	5
585		max	0	10	-.033	12	.024	1	2.039e-3	3	2820.198	12	NC	10
586		min	-.004	4	-.112	4	-.076	5	-1.908e-3	2	847.63	4	1237.76	5
587		max	0	10	-.035	12	.022	1	1.924e-3	3	2694.284	12	NC	10
588		min	-.003	4	-.117	4	-.084	5	-1.788e-3	2	809.785	4	1112.123	5
589		max	0	10	-.035	12	.02	1	1.81e-3	3	2654.451	12	NC	10
590		min	-.003	4	-.118	4	-.089	5	-1.667e-3	2	797.813	4	1049.156	5
591		max	0	10	-.035	12	.018	1	1.695e-3	3	2694.284	12	NC	10
592		min	-.003	4	-.116	4	-.09	5	-1.547e-3	2	809.785	4	1035.216	5
593		max	0	10	-.033	12	.015	1	1.58e-3	3	2820.198	12	NC	9
594		min	-.002	4	-.111	4	-.088	5	-1.426e-3	2	847.63	4	1067.803	5
595		max	0	10	-.03	12	.011	1	1.466e-3	3	3054.126	12	NC	3
596		min	-.002	4	-.102	4	-.081	5	-1.305e-3	2	917.938	4	1154.52	5
597		max	0	10	-.027	12	.008	1	1.351e-3	3	3443.91	12	NC	2
598		min	-.002	4	-.091	4	-.071	5	-1.185e-3	2	1035.09	4	1317.434	5
599		max	0	10	-.023	12	.005	1	1.237e-3	3	4092.07	12	NC	1
600		min	-.001	4	-.076	4	-.058	5	-1.064e-3	2	1229.899	4	1607.959	5
601		max	0	10	-.018	12	.003	1	1.122e-3	3	5244.159	12	NC	1
602		min	0	4	-.06	4	-.043	5	-9.436e-4	2	1576.167	4	2154.717	5
603		max	0	10	-.012	12	.001	9	1.007e-3	3	7643.895	12	NC	1
604		min	0	4	-.041	4	-.028	5	-8.23e-4	2	2297.424	4	3359.794	5
605		max	0	10	-.006	12	0	3	1.014e-3	4	NC	3	NC	1
606		min	0	4	-.021	4	-.013	5	-7.024e-4	2	4514.793	4	7289.978	5
607		max	0	1	0	1	0	1	1.087e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.818e-4	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

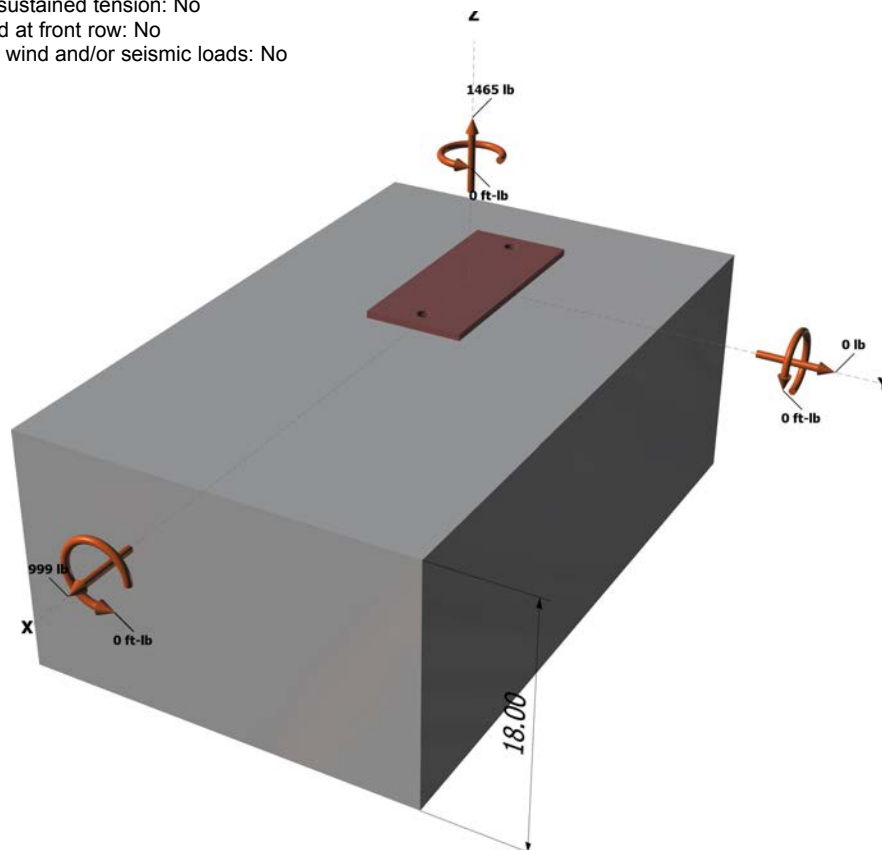
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

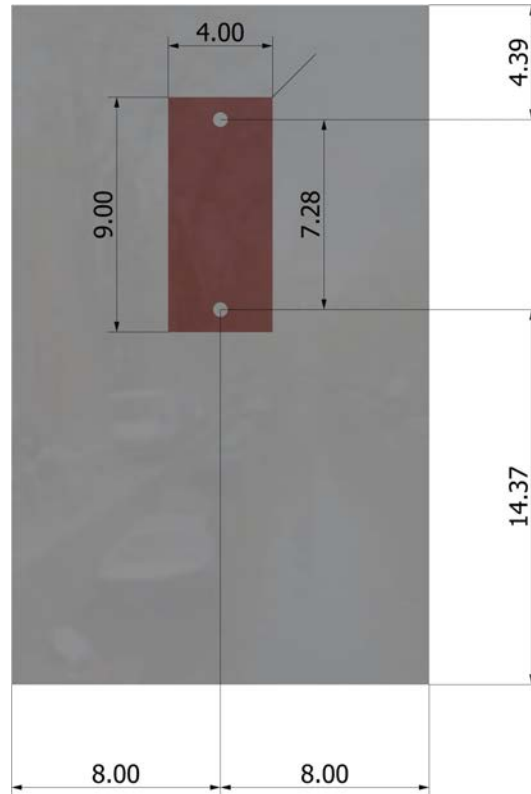
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

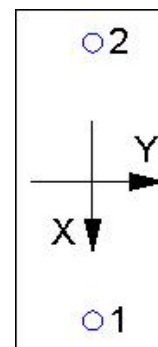
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Software  
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.