



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.39	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.705 k-ft
$M_z$ =	0.262 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>84%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

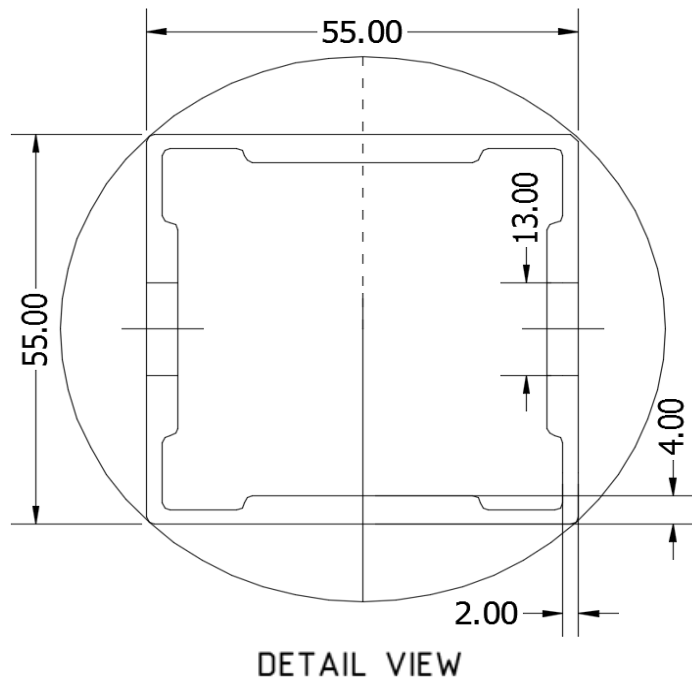
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.383 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.434 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>100%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

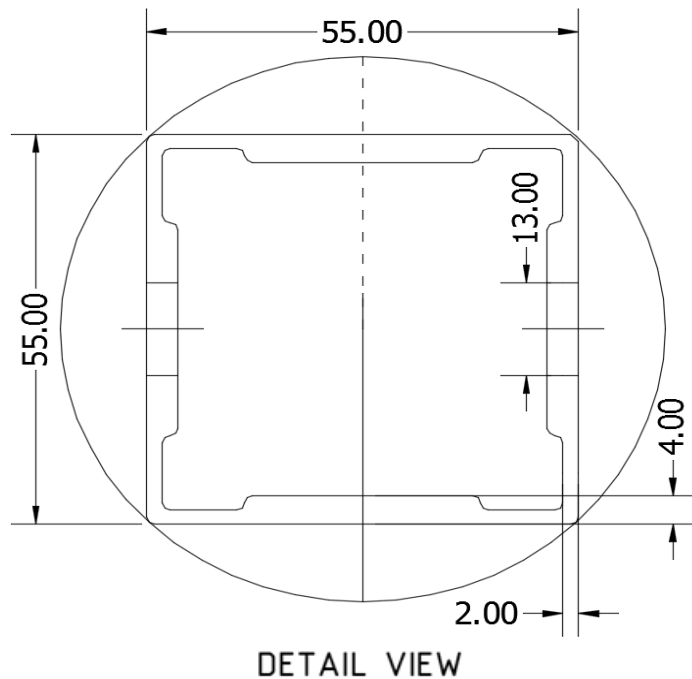
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.510 k-ft
$P_n$ =	0.626 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>39%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

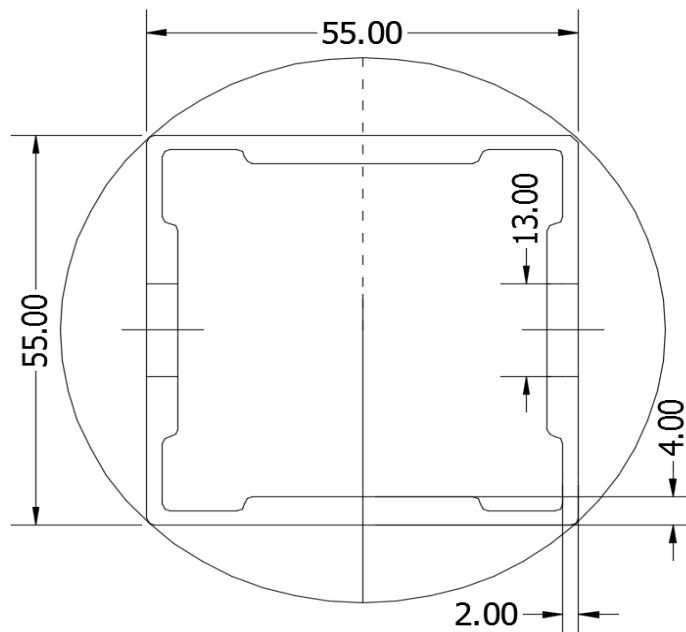
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.013 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.760 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>30%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.480 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<u>27%</u>



#### 5. FOUNDATION DESIGN CALCULATIONS

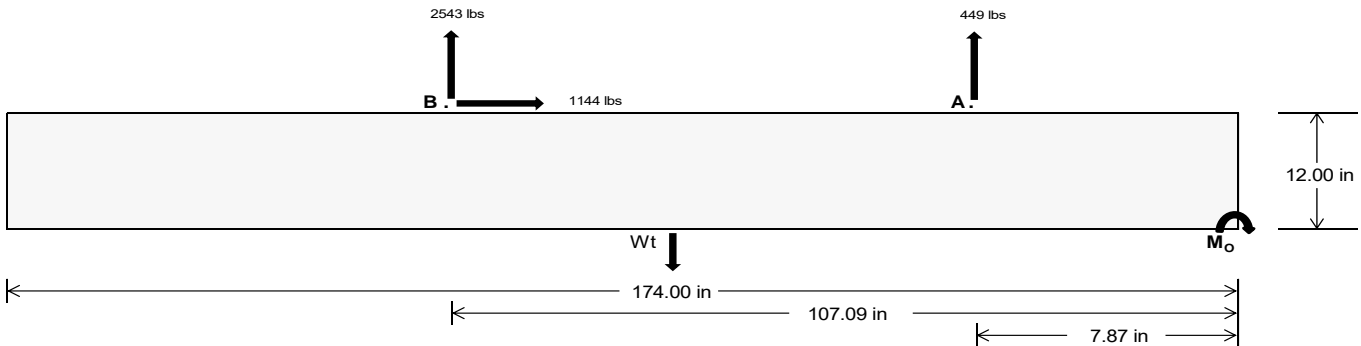
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>993.13</u>	<u>5529.35</u> k
Compressive Load =		<u>4299.45</u>	<u>4863.00</u> k
Lateral Load =		<u>337.54</u>	<u>2478.87</u> k
Moment (Weak Axis) =		<u>0.69</u>	<u>0.38</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 289537.6$  in-lbs  
Resisting Force Required = 3328.02 lbs  
S.F. = 1.67  
Weight Required = 5546.70 lbs  
Minimum Width = 32 in  
Weight Provided = 5606.67 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 174in long x 32in wide x 12in tall ballast foundation is required to resist overturning.

### Sliding

Force = 1143.68 lbs  
Friction = 0.4  
Weight Required = 2859.21 lbs  
Resisting Weight = 5606.67 lbs  
Additional Weight Required = 0 lbs

Use a 174in long x 32in wide x 12in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 1143.68 lbs  
Cohesion = 130 psf  
Area = 38.67 ft<sup>2</sup>  
Resisting = 2803.33 lbs  
Additional Weight Required = 0 lbs

Use a 174in long x 32in wide x 12in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure (Meyerhof, 1953)

$P_{ftg} = (145 \text{ pcf})(14.5 \text{ ft})(1 \text{ ft})(2.67 \text{ ft}) =$

Ballast Width			
32 in	33 in	34 in	35 in
5607 lbs	5782 lbs	5957 lbs	6132 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in
$F_A$	1493 lbs	1493 lbs	1493 lbs	1493 lbs	1467 lbs	1467 lbs	1467 lbs	1467 lbs	2098 lbs	2098 lbs	2098 lbs	2098 lbs	-449 lbs	-449 lbs	-449 lbs	-449 lbs
$F_B$	1604 lbs	1604 lbs	1604 lbs	1604 lbs	1793 lbs	1793 lbs	1793 lbs	1793 lbs	2417 lbs	2417 lbs	2417 lbs	2417 lbs	-2543 lbs	-2543 lbs	-2543 lbs	-2543 lbs
$F_V$	156 lbs	156 lbs	156 lbs	156 lbs	1020 lbs	1020 lbs	1020 lbs	1020 lbs	871 lbs	871 lbs	871 lbs	871 lbs	-1144 lbs	-1144 lbs	-1144 lbs	-1144 lbs
$P_{total}$	8704 lbs	8879 lbs	9054 lbs	9230 lbs	8866 lbs	9042 lbs	9217 lbs	9392 lbs	10121 lbs	10296 lbs	10472 lbs	10647 lbs	372 lbs	478 lbs	583 lbs	688 lbs
$M$	7006 lbs-ft	7006 lbs-ft	7006 lbs-ft	7006 lbs-ft	5651 lbs-ft	5651 lbs-ft	5651 lbs-ft	5651 lbs-ft	8914 lbs-ft	8914 lbs-ft	8914 lbs-ft	8914 lbs-ft	2439 lbs-ft	2439 lbs-ft	2439 lbs-ft	2439 lbs-ft
$e$	0.80 ft	0.79 ft	0.77 ft	0.76 ft	0.64 ft	0.63 ft	0.61 ft	0.60 ft	0.88 ft	0.87 ft	0.85 ft	0.84 ft	6.55 ft	5.11 ft	4.19 ft	3.55 ft
$L'$	12.89 ft	12.92 ft	12.95 ft	12.98 ft	13.23 ft	13.25 ft	13.27 ft	13.30 ft	12.74 ft	12.77 ft	12.80 ft	12.83 ft	1.40 ft	4.28 ft	6.13 ft	7.41 ft
$A'$	34.4 sqft	35.5 sqft	36.7 sqft	37.9 sqft	35.3 sqft	36.4 sqft	37.6 sqft	38.8 sqft	34.0 sqft	35.1 sqft	36.3 sqft	37.4 sqft	3.7 sqft	11.8 sqft	17.4 sqft	21.6 sqft
$f_{meyerhof}$	253.2 psf	249.9 psf	246.7 psf	243.8 psf	251.4 psf	248.1 psf	245.1 psf	242.2 psf	298.0 psf	293.2 psf	288.8 psf	284.6 psf	99.7 psf	40.5 psf	33.6 psf	31.8 psf

Maximum Bearing Pressure = 298 psf  
Allowable Bearing Pressure = 1500 psf

Use a 174in long x 32in wide x 12in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

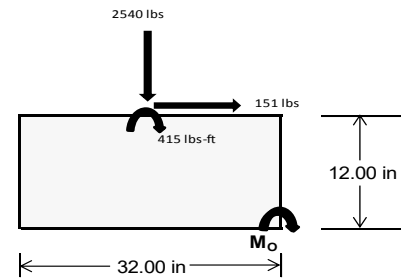
### Overturning Check

$M_o = 2821.0$  ft-lbs  
 Resisting Force Required = 2115.78 lbs  
 S.F. = 1.67  
 Weight Required = 3526.29 lbs  
 Minimum Width = **32 in**  
 Weight Provided = 5606.67 lbs

*A minimum 174in long x 32in wide x 12in tall ballast foundation is required to resist overturning.*

### Bearing Pressure (Meyerhof, 1953)

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	32 in			32 in			32 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	281 lbs	628 lbs	213 lbs	911 lbs	2540 lbs	858 lbs	106 lbs	184 lbs	38 lbs
$F_v$	212 lbs	207 lbs	215 lbs	156 lbs	151 lbs	167 lbs	212 lbs	208 lbs	213 lbs
$P_{total}$	7222 lbs	7569 lbs	7154 lbs	7518 lbs	9148 lbs	7465 lbs	2135 lbs	2213 lbs	2068 lbs
$M$	751 lbs-ft	741 lbs-ft	758 lbs-ft	566 lbs-ft	566 lbs-ft	597 lbs-ft	748 lbs-ft	738 lbs-ft	751 lbs-ft
$e$	0.10 ft	0.10 ft	0.11 ft	0.08 ft	0.06 ft	0.08 ft	0.35 ft	0.33 ft	0.36 ft
$B'$	2.46 ft	2.47 ft	2.45 ft	2.52 ft	2.54 ft	2.51 ft	1.97 ft	2.00 ft	1.94 ft
$A'$	35.7 sqft	35.8 sqft	35.6 sqft	36.5 sqft	36.9 sqft	36.3 sqft	28.5 sqft	29.0 sqft	28.1 sqft
$f_{meyerhof}$	202.6 psf	211.3 psf	201.0 psf	206.1 psf	248.1 psf	205.4 psf	74.9 psf	76.3 psf	73.5 psf



Maximum Bearing Pressure = 248 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 174in long x 32in wide x 12in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 174in long x 32in wide x 12in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.702 k
Allowable Uplift =	1.214 k
Utilization =	<u>58%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.117 k
Allowable Uplift =	4.357 k
Utilization =	<u>49%</u>



### 6.2 Strut Connections

The aluminum struts connect the front end of girder to a center section of the steel post. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.307 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>45%</u>

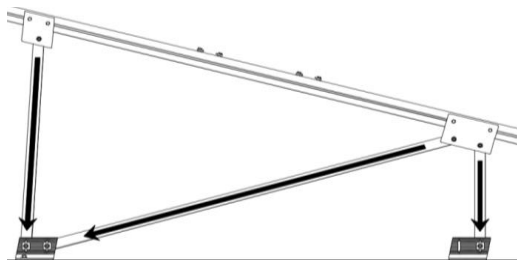
#### Rear Strut

Maximum Axial Load =	3.804 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>51%</u>

#### Diagonal Strut

Maximum Axial Load =	1.901 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>26%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	51.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.038 in
	<u>0.649 ≤ 1.038, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 102 \text{ in}$$

$$J = 0.432$$

$$282.18$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102$$

$$J = 0.432$$

$$179.449$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.0$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$C_c = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$C_c = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - F_{cy}}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{F_{cy}/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/(Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.41345$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.77788$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 13.6277 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 13.63 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 14.03 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-91.409	-91.409	0	0
2	M14	y	-91.409	-91.409	0	0
3	M15	y	-143.642	-143.642	0	0
4	M16	y	-143.642	-143.642	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	208.934	208.934	0	0
2	M14	y	160.183	160.183	0	0
3	M15	y	87.056	87.056	0	0
4	M16	y	87.056	87.056	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 72 Cell 2V 20° 130mph 30psf 8.5ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	67.474	1	1077.148	3	176.536	1	.005	14	.264	1	1.477	1
20			min	3.908	12	-828.596	1	-105.546	14	-.015	2	.005	12	-1.821	3
21		11	max	67.474	1	683.085	1	-4.004	12	.015	2	.115	1	.763	1
22			min	3.908	12	-885.379	3	-139.285	1	0	15	0	3	-.894	3
23		12	max	67.474	1	537.575	1	-2.655	12	.015	2	.051	4	.187	1
24			min	3.908	12	-693.61	3	-102.034	1	0	15	-.004	3	-.148	3
25		13	max	67.474	1	392.064	1	-1.307	12	.015	2	.024	5	.416	3
26			min	3.908	12	-501.842	3	-64.783	1	0	15	-.078	1	-.252	1
27		14	max	67.474	1	246.553	1	.23	3	.015	2	0	15	.8	3
28			min	1.919	15	-310.073	3	-30.936	4	0	15	-.121	1	-.554	1
29		15	max	67.474	1	101.042	1	9.718	1	.015	2	-.004	12	1.002	3
30			min	-8.211	5	-118.304	3	-22.765	5	0	15	-.13	1	-.718	1
31		16	max	67.474	1	73.465	3	46.969	1	.015	2	-.003	12	1.023	3
32			min	-19.355	5	-44.469	1	-20.713	5	0	15	-.103	1	-.744	1
33		17	max	67.474	1	265.234	3	84.219	1	.015	2	.001	3	.863	3
34			min	-30.5	5	-189.98	1	-18.66	5	0	15	-.072	4	-.634	1
35		18	max	67.474	1	457.003	3	121.47	1	.015	2	.056	1	.522	3
36			min	-41.645	5	-335.491	1	-16.608	5	0	15	-.079	5	-.386	1
37		19	max	67.474	1	648.771	3	158.721	1	.015	2	.188	1	0	1
38			min	-52.79	5	-481.002	1	-14.555	5	0	15	-.093	5	0	3
39	M14	1	max	52.073	4	541.678	1	-7.031	12	.012	3	.225	1	0	1
40			min	2.055	12	-524.14	3	-165.163	1	-.015	1	.013	12	0	3
41		2	max	40.928	4	396.167	1	-5.682	12	.012	3	.151	4	.426	3
42			min	2.055	12	-378.414	3	-127.912	1	-.015	1	.006	10	-.443	1
43		3	max	40.58	1	250.657	1	-4.333	12	.012	3	.089	5	.715	3
44			min	2.055	12	-232.688	3	-90.661	1	-.015	1	-.017	1	-.748	1
45		4	max	40.58	1	105.146	1	-2.985	12	.012	3	.05	5	.866	3
46			min	2.055	12	-86.961	3	-53.411	1	-.015	1	-.085	1	-.916	1
47		5	max	40.58	1	58.765	3	-.905	10	.012	3	.013	5	.879	3
48			min	-.947	5	-40.365	1	-42.527	4	-.015	1	-.118	1	-.947	1
49		6	max	40.58	1	204.491	3	21.091	1	.012	3	-.005	12	.755	3
50			min	-12.091	5	-185.876	1	-36.088	5	-.015	1	-.116	1	-.84	1
51		7	max	40.58	1	350.217	3	58.342	1	.012	3	-.004	12	.493	3
52			min	-23.236	5	-331.387	1	-34.036	5	-.015	1	-.078	1	-.596	1
53		8	max	40.58	1	495.943	3	95.592	1	.012	3	.001	10	.093	3
54			min	-34.381	5	-476.898	1	-31.983	5	-.015	1	-.09	4	-.214	1
55		9	max	40.58	1	641.67	3	132.843	1	.012	3	.103	1	.305	1
56			min	-45.526	5	-622.409	1	-29.93	5	-.015	1	-.116	5	-.444	3
57		10	max	69.264	4	787.396	3	170.094	1	.012	3	.246	1	.962	1
58			min	2.055	12	-767.92	1	-109.547	14	-.015	1	.004	12	-1.119	3
59		11	max	58.119	4	622.409	1	-3.758	12	.015	1	.151	4	.305	1
60			min	2.055	12	-641.67	3	-132.843	1	-.012	3	0	3	-.444	3
61		12	max	46.974	4	476.898	1	-2.409	12	.015	1	.087	4	.093	3
62			min	2.055	12	-495.943	3	-95.592	1	-.012	3	-.005	1	-.214	1
63		13	max	40.58	1	331.387	1	-1.061	12	.015	1	.047	5	.493	3
64			min	2.055	12	-350.217	3	-58.342	1	-.012	3	-.078	1	-.596	1
65		14	max	40.58	1	185.876	1	.601	3	.015	1	.01	5	.755	3
66			min	2.055	12	-204.491	3	-43.493	4	-.012	3	-.116	1	-.84	1
67		15	max	40.58	1	40.365	1	16.16	1	.015	1	-.004	12	.879	3
68			min	2.055	12	-58.765	3	-36.293	5	-.012	3	-.118	1	-.947	1
69		16	max	40.58	1	86.961	3	53.411	1	.015	1	-.002	12	.866	3
70			min	-6.138	5	-105.146	1	-34.24	5	-.012	3	-.085	1	-.916	1
71		17	max	40.58	1	232.688	3	90.661	1	.015	1	.003	3	.715	3
72			min	-17.283	5	-250.657	1	-32.187	5	-.012	3	-.096	4	-.748	1
73		18	max	40.58	1	378.414	3	127.912	1	.015	1	.086	1	.426	3
74			min	-28.428	5	-396.167	1	-30.135	5	-.012	3	-.12	5	-.443	1
75		19	max	40.58	1	524.14	3	165.163	1	.015	1	.225	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-39.573	5	-541.678	1	-28.082	5	-.012	3	-.147	5	0	3
77	M15	1	max	84.707	5	641.87	2	-6.942	12	.016	1	.289	4	0	2
78			min	-43.01	1	-291.739	3	-165.134	1	-.01	3	.012	12	0	3
79		2	max	73.562	5	465.917	2	-5.594	12	.016	1	.202	4	.239	3
80			min	-43.01	1	-215.077	3	-127.883	1	-.01	3	.006	12	-.523	2
81		3	max	62.417	5	289.964	2	-4.245	12	.016	1	.126	5	.406	3
82			min	-43.01	1	-138.415	3	-90.633	1	-.01	3	-.017	1	-.88	2
83		4	max	51.272	5	114.371	1	-2.896	12	.016	1	.072	5	.501	3
84			min	-43.01	1	-61.753	3	-67.377	4	-.01	3	-.085	1	-1.071	2
85		5	max	40.127	5	14.909	3	-.945	10	.016	1	.021	5	.523	3
86			min	-43.01	1	-61.942	2	-57.625	4	-.01	3	-.118	1	-1.095	2
87		6	max	28.982	5	91.57	3	21.12	1	.016	1	-.005	12	.473	3
88			min	-43.01	1	-237.895	2	-51.169	5	-.01	3	-.116	1	-.954	2
89		7	max	17.838	5	168.232	3	58.37	1	.016	1	-.004	12	.35	3
90			min	-43.01	1	-413.848	2	-49.117	5	-.01	3	-.093	4	-.648	1
91		8	max	6.693	5	244.894	3	95.621	1	.016	1	.001	10	.155	3
92			min	-43.01	1	-589.801	2	-47.064	5	-.01	3	-.124	4	-.194	1
93		9	max	-2.544	12	321.556	3	132.872	1	.016	1	.103	1	.468	2
94			min	-43.01	1	-765.754	2	-45.012	5	-.01	3	-.164	5	-.113	3
95		10	max	-2.544	12	398.218	3	170.123	1	.016	1	.287	4	1.274	2
96			min	-43.01	1	-941.707	2	-117.227	14	-.005	14	.005	12	-.453	3
97		11	max	-.38	15	765.754	2	-3.846	12	.01	3	.199	4	.468	2
98			min	-43.01	1	-321.556	3	-132.872	1	-.016	1	0	3	-.113	3
99		12	max	-2.544	12	589.801	2	-2.498	12	.01	3	.121	4	.155	3
100			min	-43.01	1	-244.894	3	-95.621	1	-.016	1	-.005	1	-.194	1
101		13	max	-2.544	12	413.848	2	-1.149	12	.01	3	.067	5	.35	3
102			min	-43.01	1	-168.232	3	-68.371	4	-.016	1	-.078	1	-.648	1
103		14	max	-2.544	12	237.895	2	.454	3	.01	3	.015	5	.473	3
104			min	-43.923	4	-91.57	3	-58.62	4	-.016	1	-.116	1	-.954	2
105		15	max	-2.544	12	61.942	2	16.131	1	.01	3	-.004	12	.523	3
106			min	-55.068	4	-14.909	3	-51.374	5	-.016	1	-.118	1	-1.095	2
107		16	max	-2.544	12	61.753	3	53.382	1	.01	3	-.002	12	.501	3
108			min	-66.213	4	-114.371	1	-49.322	5	-.016	1	-.101	4	-1.071	2
109		17	max	-2.544	12	138.415	3	90.633	1	.01	3	.003	3	.406	3
110			min	-77.358	4	-289.964	2	-47.269	5	-.016	1	-.133	4	-.88	2
111		18	max	-2.544	12	215.077	3	127.883	1	.01	3	.086	1	.239	3
112			min	-88.503	4	-465.917	2	-45.217	5	-.016	1	-.171	5	-.523	2
113		19	max	-2.544	12	291.739	3	165.134	1	.01	3	.224	1	0	2
114			min	-99.648	4	-641.87	2	-43.164	5	-.016	1	-.213	5	0	5
115	M16	1	max	80.402	5	586.223	2	-6.509	12	.012	1	.204	4	0	2
116			min	-75.03	1	-250.321	3	-159.211	1	-.012	3	.01	12	0	3
117		2	max	69.257	5	410.27	2	-5.16	12	.012	1	.136	4	.2	3
118			min	-75.03	1	-173.659	3	-121.961	1	-.012	3	.004	10	-.471	2
119		3	max	58.112	5	234.317	2	-3.811	12	.012	1	.085	5	.328	3
120			min	-75.03	1	-96.997	3	-84.71	1	-.012	3	-.04	1	-.775	2
121		4	max	46.968	5	58.364	2	-2.463	12	.012	1	.049	5	.383	3
122			min	-75.03	1	-20.335	3	-47.459	1	-.012	3	-.102	1	-.913	2
123		5	max	35.823	5	56.327	3	-.489	10	.012	1	.015	5	.366	3
124			min	-75.03	1	-117.589	2	-37.377	4	-.012	3	-.129	1	-.885	2
125		6	max	24.678	5	132.988	3	27.042	1	.012	1	-.005	12	.277	3
126			min	-75.03	1	-293.542	2	-32.495	5	-.012	3	-.121	1	-.691	2
127		7	max	13.533	5	209.65	3	64.293	1	.012	1	-.004	12	.115	3
128			min	-75.03	1	-469.495	2	-30.443	5	-.012	3	-.078	1	-.331	2
129		8	max	2.388	5	286.312	3	101.544	1	.012	1	.002	2	.201	1
130			min	-75.03	1	-645.448	2	-28.39	5	-.012	3	-.075	4	-.119	3
131		9	max	-3.936	12	362.974	3	138.794	1	.012	1	.114	1	.888	2
132			min	-75.03	1	-821.401	2	-26.337	5	-.012	3	-.1	5	-.426	3



Company : Schletter, Inc.  
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Nov 3, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-3.936	12	439.636	3	176.045	1	.012	1	.262	1	1.747	2
134			min	-75.03	1	-997.354	2	-110.799	14	-.012	3	.006	12	-.805	3
135		11	max	.572	5	821.401	2	-4.28	12	.012	3	.137	4	.888	2
136			min	-75.03	1	-362.974	3	-138.794	1	-.012	1	.001	12	-.426	3
137		12	max	-3.936	12	645.448	2	-2.931	12	.012	3	.075	4	.201	1
138			min	-75.03	1	-286.312	3	-101.544	1	-.012	1	-.003	3	-.119	3
139		13	max	-3.936	12	469.495	2	-1.583	12	.012	3	.037	5	.115	3
140			min	-75.03	1	-209.65	3	-64.293	1	-.012	1	-.078	1	-.331	2
141		14	max	-3.936	12	293.542	2	-.232	3	.012	3	.003	5	.277	3
142			min	-75.03	1	-132.988	3	-41.551	4	-.012	1	-.121	1	-.691	2
143		15	max	-3.936	12	117.589	2	10.209	1	.012	3	-.004	12	.366	3
144			min	-75.03	1	-56.327	3	-33.388	5	-.012	1	-.129	1	-.885	2
145		16	max	-3.936	12	20.335	3	47.459	1	.012	3	-.003	12	.383	3
146			min	-75.03	1	-58.364	2	-31.335	5	-.012	1	-.102	1	-.913	2
147		17	max	-3.936	12	96.997	3	84.71	1	.012	3	0	3	.328	3
148			min	-82.844	4	-234.317	2	-29.282	5	-.012	1	-.098	4	-.775	2
149		18	max	-3.936	12	173.659	3	121.961	1	.012	3	.058	1	.2	3
150			min	-93.989	4	-410.27	2	-27.23	5	-.012	1	-.116	5	-.471	2
151		19	max	-3.936	12	250.321	3	159.211	1	.012	3	.191	1	0	2
152			min	-105.134	4	-586.223	2	-25.177	5	-.012	1	-.14	5	0	5
153	M2	1	max	1119.129	1	2.213	4	.796	1	0	3	0	3	0	1
154			min	-1196.132	3	.544	15	-54.587	4	0	1	0	1	0	1
155		2	max	1119.545	1	2.204	4	.796	1	0	3	0	1	0	15
156			min	-1195.82	3	.542	15	-54.948	4	0	1	-.015	4	0	4
157		3	max	1119.961	1	2.195	4	.796	1	0	3	0	1	0	15
158			min	-1195.508	3	.54	15	-55.308	4	0	1	-.031	4	-.001	4
159		4	max	1120.377	1	2.186	4	.796	1	0	3	0	1	0	15
160			min	-1195.196	3	.538	15	-55.668	4	0	1	-.046	4	-.002	4
161		5	max	1120.793	1	2.178	4	.796	1	0	3	0	1	0	15
162			min	-1194.884	3	.536	15	-56.029	4	0	1	-.062	4	-.002	4
163		6	max	1121.209	1	2.169	4	.796	1	0	3	.001	1	0	15
164			min	-1194.572	3	.534	15	-56.389	4	0	1	-.078	4	-.003	4
165		7	max	1121.625	1	2.16	4	.796	1	0	3	.001	1	0	15
166			min	-1194.26	3	.532	15	-56.75	4	0	1	-.094	4	-.004	4
167		8	max	1122.04	1	2.152	4	.796	1	0	3	.002	1	-.001	15
168			min	-1193.948	3	.529	15	-57.11	4	0	1	-.11	4	-.004	4
169		9	max	1122.456	1	2.143	4	.796	1	0	3	.002	1	-.001	15
170			min	-1193.637	3	.527	15	-57.471	4	0	1	-.126	4	-.005	4
171		10	max	1122.872	1	2.134	4	.796	1	0	3	.002	1	-.001	15
172			min	-1193.325	3	.525	15	-57.831	4	0	1	-.142	4	-.005	4
173		11	max	1123.288	1	2.125	4	.796	1	0	3	.002	1	-.001	15
174			min	-1193.013	3	.523	15	-58.192	4	0	1	-.158	4	-.006	4
175		12	max	1123.704	1	2.117	4	.796	1	0	3	.002	1	-.002	15
176			min	-1192.701	3	.521	15	-58.552	4	0	1	-.174	4	-.007	4
177		13	max	1124.12	1	2.108	4	.796	1	0	3	.003	1	-.002	15
178			min	-1192.389	3	.519	15	-58.913	4	0	1	-.191	4	-.007	4
179		14	max	1124.536	1	2.099	4	.796	1	0	3	.003	1	-.002	15
180			min	-1192.077	3	.517	15	-59.273	4	0	1	-.207	4	-.008	4
181		15	max	1124.952	1	2.091	4	.796	1	0	3	.003	1	-.002	15
182			min	-1191.765	3	.515	15	-59.634	4	0	1	-.224	4	-.008	4
183		16	max	1125.367	1	2.082	4	.796	1	0	3	.003	1	-.002	15
184			min	-1191.453	3	.513	15	-59.994	4	0	1	-.241	4	-.009	4
185		17	max	1125.783	1	2.073	4	.796	1	0	3	.004	1	-.002	15
186			min	-1191.141	3	.511	15	-60.355	4	0	1	-.258	4	-.01	4
187		18	max	1126.199	1	2.064	4	.796	1	0	3	.004	1	-.003	15
188			min	-1190.829	3	.509	15	-60.715	4	0	1	-.275	4	-.01	4
189		19	max	1126.615	1	2.056	4	.796	1	0	3	.004	1	-.003	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1190.517	3	.507	15	-61.076	4	0	1	-.292	4	-.011	4
191	M3	1	max	487.197	2	9.134	4	.188	1	0	0	1	.011	4
192		min	-624.065	3	2.161	15	-3.255	5	0	4	-.005	4	.003	15
193		2	max	487.026	2	8.259	4	.188	1	0	0	1	.007	4
194		min	-624.193	3	1.956	15	-2.647	5	0	4	-.006	4	.001	12
195		3	max	486.856	2	7.385	4	.188	1	0	0	1	.003	2
196		min	-624.321	3	1.75	15	-2.038	5	0	4	-.007	4	0	3
197		4	max	486.685	2	6.51	4	.188	1	0	0	1	0	2
198		min	-624.449	3	1.544	15	-1.429	5	0	4	-.008	5	-.002	3
199		5	max	486.515	2	5.636	4	.188	1	0	0	1	0	15
200		min	-624.576	3	1.339	15	-.82	5	0	4	-.009	5	-.003	3
201		6	max	486.345	2	4.762	4	.188	1	0	0	1	-.001	15
202		min	-624.704	3	1.133	15	-.212	5	0	4	-.009	5	-.006	6
203		7	max	486.174	2	3.887	4	.457	4	0	0	1	-.002	15
204		min	-624.832	3	.928	15	.01	12	0	4	-.009	5	-.008	6
205		8	max	486.004	2	3.013	4	1.066	4	0	0	1	-.002	15
206		min	-624.96	3	.722	15	.01	12	0	4	-.009	5	-.009	6
207		9	max	485.834	2	2.138	4	1.674	4	0	0	1	-.002	15
208		min	-625.087	3	.517	15	.01	12	0	4	-.008	5	-.011	6
209		10	max	485.663	2	1.264	4	2.283	4	0	0	1	-.003	15
210		min	-625.215	3	.311	15	.01	12	0	4	-.007	5	-.011	6
211		11	max	485.493	2	.426	2	2.892	4	0	0	1	-.003	15
212		min	-625.343	3	.003	3	.01	12	0	4	-.006	5	-.012	6
213		12	max	485.323	2	-.1	15	3.5	4	0	0	1	-.003	15
214		min	-625.471	3	-.508	3	.01	12	0	4	-.004	5	-.012	6
215		13	max	485.152	2	-.306	15	4.109	4	0	0	1	-.003	15
216		min	-625.599	3	-1.361	6	.01	12	0	4	-.003	5	-.011	6
217		14	max	484.982	2	-.511	15	4.718	4	0	0	1	-.002	15
218		min	-625.726	3	-2.235	6	.01	12	0	4	0	5	-.01	6
219		15	max	484.812	2	-.717	15	5.327	4	0	0	4	-.002	15
220		min	-625.854	3	-3.11	6	.01	12	0	4	0	12	-.009	6
221		16	max	484.641	2	-.922	15	5.935	4	0	0	4	-.002	15
222		min	-625.982	3	-3.984	6	.01	12	0	4	0	12	-.008	6
223		17	max	484.471	2	-1.128	15	6.544	4	0	0	4	-.001	15
224		min	-626.11	3	-4.859	6	.01	12	0	4	0	12	-.005	6
225		18	max	484.301	2	-1.333	15	7.153	4	0	0	4	0	15
226		min	-626.237	3	-5.733	6	.01	12	0	4	0	12	-.003	6
227		19	max	484.13	2	-1.539	15	7.761	4	0	0	4	0	1
228		min	-626.365	3	-6.607	6	.01	12	0	4	0	12	0	1
229	M4	1	max	1185.799	1	0	1	-.574	12	0	1	.009	4	0
230		min	-222.742	3	0	1	-258.303	4	0	1	0	12	0	1
231		2	max	1185.969	1	0	1	-.574	12	0	1	0	12	0
232		min	-222.614	3	0	1	-258.45	4	0	1	-.021	4	0	1
233		3	max	1186.139	1	0	1	-.574	12	0	1	0	12	0
234		min	-222.486	3	0	1	-258.598	4	0	1	-.051	4	0	1
235		4	max	1186.31	1	0	1	-.574	12	0	1	0	12	0
236		min	-222.358	3	0	1	-258.745	4	0	1	-.08	4	0	1
237		5	max	1186.48	1	0	1	-.574	12	0	1	0	12	0
238		min	-222.231	3	0	1	-258.893	4	0	1	-.11	4	0	1
239		6	max	1186.65	1	0	1	-.574	12	0	1	0	12	0
240		min	-222.103	3	0	1	-259.041	4	0	1	-.14	4	0	1
241		7	max	1186.821	1	0	1	-.574	12	0	1	0	12	0
242		min	-221.975	3	0	1	-259.188	4	0	1	-.17	4	0	1
243		8	max	1186.991	1	0	1	-.574	12	0	1	0	12	0
244		min	-221.847	3	0	1	-259.336	4	0	1	-.199	4	0	1
245		9	max	1187.161	1	0	1	-.574	12	0	1	0	12	0
246		min	-221.72	3	0	1	-259.484	4	0	1	-.229	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1187.332	1	0	1	-.574	12	0	1	0	12	0	1
248		min	-221.592	3	0	1	-259.631	4	0	1	-.259	4	0	1
249	11	max	1187.502	1	0	1	-.574	12	0	1	0	12	0	1
250		min	-221.464	3	0	1	-259.779	4	0	1	-.289	4	0	1
251	12	max	1187.672	1	0	1	-.574	12	0	1	0	12	0	1
252		min	-221.336	3	0	1	-259.926	4	0	1	-.319	4	0	1
253	13	max	1187.843	1	0	1	-.574	12	0	1	0	12	0	1
254		min	-221.209	3	0	1	-260.074	4	0	1	-.348	4	0	1
255	14	max	1188.013	1	0	1	-.574	12	0	1	0	12	0	1
256		min	-221.081	3	0	1	-260.222	4	0	1	-.378	4	0	1
257	15	max	1188.183	1	0	1	-.574	12	0	1	0	12	0	1
258		min	-220.953	3	0	1	-260.369	4	0	1	-.408	4	0	1
259	16	max	1188.354	1	0	1	-.574	12	0	1	0	12	0	1
260		min	-220.825	3	0	1	-260.517	4	0	1	-.438	4	0	1
261	17	max	1188.524	1	0	1	-.574	12	0	1	0	12	0	1
262		min	-220.698	3	0	1	-260.665	4	0	1	-.468	4	0	1
263	18	max	1188.695	1	0	1	-.574	12	0	1	-.001	12	0	1
264		min	-220.57	3	0	1	-260.812	4	0	1	-.498	4	0	1
265	19	max	1188.865	1	0	1	-.574	12	0	1	-.001	12	0	1
266		min	-220.442	3	0	1	-260.96	4	0	1	-.528	4	0	1
267	M6	1	max	3472.558	1	2.552	2	0	1	0	0	4	0	1
268		min	-3803.676	3	.192	3	-55.167	4	0	4	0	1	0	1
269	2	max	3472.974	1	2.545	2	0	1	0	1	0	1	0	3
270		min	-3803.365	3	.187	3	-55.528	4	0	4	-.016	4	0	2
271	3	max	3473.39	1	2.539	2	0	1	0	1	0	1	0	3
272		min	-3803.053	3	.182	3	-55.888	4	0	4	-.031	4	-.001	2
273	4	max	3473.806	1	2.532	2	0	1	0	1	0	1	0	3
274		min	-3802.741	3	.177	3	-56.249	4	0	4	-.047	4	-.002	2
275	5	max	3474.222	1	2.525	2	0	1	0	1	0	1	0	3
276		min	-3802.429	3	.172	3	-56.609	4	0	4	-.063	4	-.003	2
277	6	max	3474.638	1	2.518	2	0	1	0	1	0	1	0	3
278		min	-3802.117	3	.167	3	-56.969	4	0	4	-.079	4	-.004	2
279	7	max	3475.054	1	2.511	2	0	1	0	1	0	1	0	3
280		min	-3801.805	3	.162	3	-57.33	4	0	4	-.095	4	-.004	2
281	8	max	3475.469	1	2.505	2	0	1	0	1	0	1	0	3
282		min	-3801.493	3	.157	3	-57.69	4	0	4	-.111	4	-.005	2
283	9	max	3475.885	1	2.498	2	0	1	0	1	0	1	0	3
284		min	-3801.181	3	.151	3	-58.051	4	0	4	-.127	4	-.006	2
285	10	max	3476.301	1	2.491	2	0	1	0	1	0	1	0	3
286		min	-3800.869	3	.146	3	-58.411	4	0	4	-.143	4	-.006	2
287	11	max	3476.717	1	2.484	2	0	1	0	1	0	1	0	3
288		min	-3800.557	3	.141	3	-58.772	4	0	4	-.16	4	-.007	2
289	12	max	3477.133	1	2.477	2	0	1	0	1	0	1	0	3
290		min	-3800.245	3	.136	3	-59.132	4	0	4	-.176	4	-.008	2
291	13	max	3477.549	1	2.471	2	0	1	0	1	0	1	0	3
292		min	-3799.934	3	.131	3	-59.493	4	0	4	-.193	4	-.008	2
293	14	max	3477.965	1	2.464	2	0	1	0	1	0	1	0	3
294		min	-3799.622	3	.126	3	-59.853	4	0	4	-.21	4	-.009	2
295	15	max	3478.381	1	2.457	2	0	1	0	1	0	1	0	3
296		min	-3799.31	3	.121	3	-60.214	4	0	4	-.226	4	-.01	2
297	16	max	3478.796	1	2.45	2	0	1	0	1	0	1	0	3
298		min	-3798.998	3	.116	3	-60.574	4	0	4	-.243	4	-.011	2
299	17	max	3479.212	1	2.443	2	0	1	0	1	0	1	0	3
300		min	-3798.686	3	.111	3	-60.935	4	0	4	-.26	4	-.011	2
301	18	max	3479.628	1	2.437	2	0	1	0	1	0	1	0	3
302		min	-3798.374	3	.106	3	-61.295	4	0	4	-.278	4	-.012	2
303	19	max	3480.044	1	2.43	2	0	1	0	1	0	1	0	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-3798.062	3	.1	3	-61.656	4	0	4	-.295	4	-.013	2
305	M7	1	max	1759.943	2	9.139	6	0	1	0	0	1	.013	2
306		min	-1898.363	3	2.144	15	-3.465	5	0	4	-.005	4	0	3
307		2	max	1759.773	2	8.264	6	0	1	0	0	1	.009	2
308		min	-1898.491	3	1.939	15	-2.856	5	0	4	-.006	4	-.001	3
309		3	max	1759.603	2	7.39	6	0	1	0	0	1	.006	2
310		min	-1898.618	3	1.733	15	-2.248	5	0	4	-.008	4	-.003	3
311		4	max	1759.432	2	6.515	6	0	1	0	0	1	.003	2
312		min	-1898.746	3	1.528	15	-1.639	5	0	4	-.009	4	-.005	3
313		5	max	1759.262	2	5.641	6	0	1	0	0	1	0	2
314		min	-1898.874	3	1.322	15	-1.03	5	0	4	-.009	4	-.006	3
315		6	max	1759.092	2	4.766	6	0	1	0	0	1	-.001	15
316		min	-1899.002	3	1.117	15	-.422	5	0	4	-.01	4	-.007	3
317		7	max	1758.921	2	3.892	6	.201	4	0	0	1	-.002	15
318		min	-1899.129	3	.911	15	0	1	0	4	-.01	4	-.008	3
319		8	max	1758.751	2	3.017	6	.81	4	0	0	1	-.002	15
320		min	-1899.257	3	.706	15	0	1	0	4	-.009	4	-.009	4
321		9	max	1758.581	2	2.157	2	1.419	4	0	0	1	-.002	15
322		min	-1899.385	3	.441	12	0	1	0	4	-.009	4	-.011	4
323		10	max	1758.41	2	1.475	2	2.027	4	0	0	1	-.003	15
324		min	-1899.513	3	.087	3	0	1	0	4	-.008	4	-.011	4
325		11	max	1758.24	2	.794	2	2.636	4	0	0	1	-.003	15
326		min	-1899.64	3	-.424	3	0	1	0	4	-.007	4	-.012	4
327		12	max	1758.07	2	.112	2	3.245	4	0	0	1	-.003	15
328		min	-1899.768	3	-.935	3	0	1	0	4	-.005	4	-.012	4
329		13	max	1757.899	2	-.322	15	3.854	4	0	0	1	-.003	15
330		min	-1899.896	3	-1.446	3	0	1	0	4	-.004	4	-.011	4
331		14	max	1757.729	2	-.528	15	4.462	4	0	0	1	-.002	15
332		min	-1900.024	3	-2.229	4	0	1	0	4	-.002	4	-.01	4
333		15	max	1757.559	2	-.733	15	5.071	4	0	0	5	-.002	15
334		min	-1900.151	3	-3.104	4	0	1	0	4	0	1	-.009	4
335		16	max	1757.388	2	-.939	15	5.68	4	0	.003	4	-.002	15
336		min	-1900.279	3	-3.978	4	0	1	0	4	0	1	-.008	4
337		17	max	1757.218	2	-1.144	15	6.288	4	0	.006	4	-.001	15
338		min	-1900.407	3	-4.853	4	0	1	0	4	0	1	-.005	4
339		18	max	1757.048	2	-1.35	15	6.897	4	0	.009	4	0	15
340		min	-1900.535	3	-5.727	4	0	1	0	4	0	1	-.003	4
341		19	max	1756.877	2	-1.555	15	7.506	4	0	.012	4	0	1
342		min	-1900.662	3	-6.601	4	0	1	0	4	0	1	0	1
343	M8	1	max	3304.203	1	0	1	0	1	0	.007	4	0	1
344		min	-766.246	3	0	1	-249.034	4	0	1	0	1	0	1
345		2	max	3304.374	1	0	1	0	1	0	0	1	0	1
346		min	-766.119	3	0	1	-249.182	4	0	1	-.021	4	0	1
347		3	max	3304.544	1	0	1	0	1	0	0	1	0	1
348		min	-765.991	3	0	1	-249.329	4	0	1	-.05	4	0	1
349		4	max	3304.714	1	0	1	0	1	0	0	1	0	1
350		min	-765.863	3	0	1	-249.477	4	0	1	-.079	4	0	1
351		5	max	3304.885	1	0	1	0	1	0	0	1	0	1
352		min	-765.735	3	0	1	-249.624	4	0	1	-.107	4	0	1
353		6	max	3305.055	1	0	1	0	1	0	0	1	0	1
354		min	-765.608	3	0	1	-249.772	4	0	1	-.136	4	0	1
355		7	max	3305.225	1	0	1	0	1	0	0	1	0	1
356		min	-765.48	3	0	1	-249.92	4	0	1	-.165	4	0	1
357		8	max	3305.396	1	0	1	0	1	0	0	1	0	1
358		min	-765.352	3	0	1	-250.067	4	0	1	-.193	4	0	1
359		9	max	3305.566	1	0	1	0	1	0	0	1	0	1
360		min	-765.224	3	0	1	-250.215	4	0	1	-.222	4	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3305.736	1	0	1	0	1	0	1	0	1	0	1
362			min	-765.096	3	0	1	-250.363	4	0	1	-.251	4	0	1
363		11	max	3305.907	1	0	1	0	1	0	1	0	1	0	1
364			min	-764.969	3	0	1	-250.51	4	0	1	-.279	4	0	1
365		12	max	3306.077	1	0	1	0	1	0	1	0	1	0	1
366			min	-764.841	3	0	1	-250.658	4	0	1	-.308	4	0	1
367		13	max	3306.247	1	0	1	0	1	0	1	0	1	0	1
368			min	-764.713	3	0	1	-250.806	4	0	1	-.337	4	0	1
369		14	max	3306.418	1	0	1	0	1	0	1	0	1	0	1
370			min	-764.585	3	0	1	-250.953	4	0	1	-.366	4	0	1
371		15	max	3306.588	1	0	1	0	1	0	1	0	1	0	1
372			min	-764.458	3	0	1	-251.101	4	0	1	-.395	4	0	1
373		16	max	3306.758	1	0	1	0	1	0	1	0	1	0	1
374			min	-764.33	3	0	1	-251.248	4	0	1	-.424	4	0	1
375		17	max	3306.929	1	0	1	0	1	0	1	0	1	0	1
376			min	-764.202	3	0	1	-251.396	4	0	1	-.452	4	0	1
377		18	max	3307.099	1	0	1	0	1	0	1	0	1	0	1
378			min	-764.074	3	0	1	-251.544	4	0	1	-.481	4	0	1
379		19	max	3307.269	1	0	1	0	1	0	1	0	1	0	1
380			min	-763.947	3	0	1	-251.691	4	0	1	-.51	4	0	1
381	M10	1	max	1119.129	1	2.103	6	-.045	12	0	1	0	4	0	1
382			min	-1196.132	3	.47	15	-55.008	4	0	5	0	3	0	1
383		2	max	1119.545	1	2.094	6	-.045	12	0	1	0	10	0	15
384			min	-1195.82	3	.468	15	-55.369	4	0	5	-.015	4	0	6
385		3	max	1119.961	1	2.085	6	-.045	12	0	1	0	12	0	15
386			min	-1195.508	3	.466	15	-55.729	4	0	5	-.031	4	-.001	6
387		4	max	1120.377	1	2.077	6	-.045	12	0	1	0	12	0	15
388			min	-1195.196	3	.464	15	-56.09	4	0	5	-.047	4	-.002	6
389		5	max	1120.793	1	2.068	6	-.045	12	0	1	0	12	0	15
390			min	-1194.884	3	.462	15	-56.45	4	0	5	-.062	4	-.002	6
391		6	max	1121.209	1	2.059	6	-.045	12	0	1	0	12	0	15
392			min	-1194.572	3	.46	15	-56.811	4	0	5	-.078	4	-.003	6
393		7	max	1121.625	1	2.051	6	-.045	12	0	1	0	12	0	15
394			min	-1194.26	3	.458	15	-57.171	4	0	5	-.094	4	-.003	6
395		8	max	1122.04	1	2.042	6	-.045	12	0	1	0	12	0	15
396			min	-1193.948	3	.456	15	-57.532	4	0	5	-.11	4	-.004	6
397		9	max	1122.456	1	2.033	6	-.045	12	0	1	0	12	-.001	15
398			min	-1193.637	3	.454	15	-57.892	4	0	5	-.127	4	-.005	6
399		10	max	1122.872	1	2.024	6	-.045	12	0	1	0	12	-.001	15
400			min	-1193.325	3	.452	15	-58.253	4	0	5	-.143	4	-.005	6
401		11	max	1123.288	1	2.016	6	-.045	12	0	1	0	12	-.001	15
402			min	-1193.013	3	.45	15	-58.613	4	0	5	-.159	4	-.006	6
403		12	max	1123.704	1	2.007	6	-.045	12	0	1	0	12	-.001	15
404			min	-1192.701	3	.448	15	-58.974	4	0	5	-.176	4	-.006	6
405		13	max	1124.12	1	1.998	6	-.045	12	0	1	0	12	-.002	15
406			min	-1192.389	3	.446	15	-59.334	4	0	5	-.192	4	-.007	6
407		14	max	1124.536	1	1.99	6	-.045	12	0	1	0	12	-.002	15
408			min	-1192.077	3	.444	15	-59.695	4	0	5	-.209	4	-.007	6
409		15	max	1124.952	1	1.981	6	-.045	12	0	1	0	12	-.002	15
410			min	-1191.765	3	.442	15	-60.055	4	0	5	-.226	4	-.008	6
411		16	max	1125.367	1	1.972	6	-.045	12	0	1	0	12	-.002	15
412			min	-1191.453	3	.44	15	-60.416	4	0	5	-.243	4	-.009	6
413		17	max	1125.783	1	1.963	6	-.045	12	0	1	0	12	-.002	15
414			min	-1191.141	3	.437	15	-60.776	4	0	5	-.26	4	-.009	6
415		18	max	1126.199	1	1.955	6	-.045	12	0	1	0	12	-.002	15
416			min	-1190.829	3	.435	15	-61.137	4	0	5	-.277	4	-.01	6
417		19	max	1126.615	1	1.946	6	-.045	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1190.517	3	.433	15	-61.497	4	0	5	-.294	4	-.01	6
419	M11	1	max	487.197	2	9.069	6	-.01	12	0	1	0	12	.01	6
420			min	-624.065	3	2.118	15	-3.294	4	0	4	-.005	4	.002	15
421		2	max	487.026	2	8.194	6	-.01	12	0	1	0	12	.006	2
422			min	-624.193	3	1.912	15	-2.686	4	0	4	-.006	4	.001	15
423		3	max	486.856	2	7.32	6	-.01	12	0	1	0	12	.003	2
424			min	-624.321	3	1.706	15	-2.077	4	0	4	-.007	4	0	3
425		4	max	486.685	2	6.446	6	-.01	12	0	1	0	12	0	2
426			min	-624.449	3	1.501	15	-1.468	4	0	4	-.008	4	-.002	3
427		5	max	486.515	2	5.571	6	-.01	12	0	1	0	12	0	15
428			min	-624.576	3	1.295	15	-.859	4	0	4	-.009	4	-.004	4
429		6	max	486.345	2	4.697	6	-.01	12	0	1	0	12	-.002	15
430			min	-624.704	3	1.09	15	-.251	4	0	4	-.009	4	-.006	4
431		7	max	486.174	2	3.822	6	.377	5	0	1	0	12	-.002	15
432			min	-624.832	3	.884	15	-.188	1	0	4	-.009	4	-.008	4
433		8	max	486.004	2	2.948	6	.985	5	0	1	0	12	-.002	15
434			min	-624.96	3	.679	15	-.188	1	0	4	-.009	4	-.01	4
435		9	max	485.834	2	2.073	6	1.594	5	0	1	0	12	-.003	15
436			min	-625.087	3	.473	15	-.188	1	0	4	-.008	4	-.011	4
437		10	max	485.663	2	1.199	6	2.203	5	0	1	0	12	-.003	15
438			min	-625.215	3	.268	15	-.188	1	0	4	-.007	4	-.012	4
439		11	max	485.493	2	.426	2	2.812	5	0	1	0	12	-.003	15
440			min	-625.343	3	.003	3	-.188	1	0	4	-.006	4	-.012	4
441		12	max	485.323	2	-.144	15	3.42	5	0	1	0	12	-.003	15
442			min	-625.471	3	-.551	4	-.188	1	0	4	-.005	4	-.012	4
443		13	max	485.152	2	-.349	15	4.029	5	0	1	0	12	-.003	15
444			min	-625.599	3	-1.426	4	-.188	1	0	4	-.003	4	-.012	4
445		14	max	484.982	2	-.555	15	4.638	5	0	1	0	12	-.003	15
446			min	-625.726	3	-2.3	4	-.188	1	0	4	-.001	1	-.011	4
447		15	max	484.812	2	-.76	15	5.246	5	0	1	.002	5	-.002	15
448			min	-625.854	3	-3.175	4	-.188	1	0	4	-.001	1	-.009	4
449		16	max	484.641	2	-.966	15	5.855	5	0	1	.004	5	-.002	15
450			min	-625.982	3	-4.049	4	-.188	1	0	4	-.001	1	-.008	4
451		17	max	484.471	2	-1.171	15	6.464	5	0	1	.007	5	-.001	15
452			min	-626.11	3	-4.924	4	-.188	1	0	4	-.001	1	-.005	4
453		18	max	484.301	2	-1.377	15	7.072	5	0	1	.01	5	0	15
454			min	-626.237	3	-5.798	4	-.188	1	0	4	-.002	1	-.003	4
455		19	max	484.13	2	-1.582	15	7.681	5	0	1	.014	5	0	1
456			min	-626.365	3	-6.672	4	-.188	1	0	4	-.002	1	0	1
457	M12	1	max	1185.799	1	0	1	11.212	1	0	1	.008	5	0	1
458			min	-222.742	3	0	1	-252.336	4	0	1	0	1	0	1
459		2	max	1185.969	1	0	1	11.212	1	0	1	0	1	0	1
460			min	-222.614	3	0	1	-252.484	4	0	1	-.021	4	0	1
461		3	max	1186.139	1	0	1	11.212	1	0	1	.002	1	0	1
462			min	-222.486	3	0	1	-252.631	4	0	1	-.05	4	0	1
463		4	max	1186.31	1	0	1	11.212	1	0	1	.003	1	0	1
464			min	-222.358	3	0	1	-252.779	4	0	1	-.079	4	0	1
465		5	max	1186.48	1	0	1	11.212	1	0	1	.004	1	0	1
466			min	-222.231	3	0	1	-252.927	4	0	1	-.108	4	0	1
467		6	max	1186.65	1	0	1	11.212	1	0	1	.005	1	0	1
468			min	-222.103	3	0	1	-253.074	4	0	1	-.137	4	0	1
469		7	max	1186.821	1	0	1	11.212	1	0	1	.007	1	0	1
470			min	-221.975	3	0	1	-253.222	4	0	1	-.166	4	0	1
471		8	max	1186.991	1	0	1	11.212	1	0	1	.008	1	0	1
472			min	-221.847	3	0	1	-253.37	4	0	1	-.195	4	0	1
473		9	max	1187.161	1	0	1	11.212	1	0	1	.009	1	0	1
474			min	-221.72	3	0	1	-253.517	4	0	1	-.224	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1187.332	1	0	1	11.212	1	0	1	.011	1	0	1
476			min	-221.592	3	0	1	-253.665	4	0	1	-.253	4	0	1
477		11	max	1187.502	1	0	1	11.212	1	0	1	.012	1	0	1
478			min	-221.464	3	0	1	-253.812	4	0	1	-.282	4	0	1
479		12	max	1187.672	1	0	1	11.212	1	0	1	.013	1	0	1
480			min	-221.336	3	0	1	-253.96	4	0	1	-.312	4	0	1
481		13	max	1187.843	1	0	1	11.212	1	0	1	.014	1	0	1
482			min	-221.209	3	0	1	-254.108	4	0	1	-.341	4	0	1
483		14	max	1188.013	1	0	1	11.212	1	0	1	.016	1	0	1
484			min	-221.081	3	0	1	-254.255	4	0	1	-.37	4	0	1
485		15	max	1188.183	1	0	1	11.212	1	0	1	.017	1	0	1
486			min	-220.953	3	0	1	-254.403	4	0	1	-.399	4	0	1
487		16	max	1188.354	1	0	1	11.212	1	0	1	.018	1	0	1
488			min	-220.825	3	0	1	-254.551	4	0	1	-.428	4	0	1
489		17	max	1188.524	1	0	1	11.212	1	0	1	.02	1	0	1
490			min	-220.698	3	0	1	-254.698	4	0	1	-.458	4	0	1
491		18	max	1188.695	1	0	1	11.212	1	0	1	.021	1	0	1
492			min	-220.57	3	0	1	-254.846	4	0	1	-.487	4	0	1
493		19	max	1188.865	1	0	1	11.212	1	0	1	.022	1	0	1
494			min	-220.442	3	0	1	-254.994	4	0	1	-.516	4	0	1
495	M1	1	max	158.726	1	648.729	3	52.754	5	0	1	.188	1	0	15
496			min	-14.555	5	-478.854	1	-67.373	1	0	3	-.093	5	-.015	2
497		2	max	159.302	1	647.542	3	54.214	5	0	1	.147	1	.283	1
498			min	-14.286	5	-480.437	1	-67.373	1	0	3	-.06	5	-.405	3
499		3	max	402.564	3	570.986	1	2.491	5	0	3	.105	1	.57	1
500			min	-262.698	2	-487.301	3	-66.795	1	0	1	-.027	5	-.794	3
501		4	max	402.996	3	569.403	1	3.951	5	0	3	.063	1	.217	1
502			min	-262.122	2	-488.489	3	-66.795	1	0	1	-.025	5	-.491	3
503		5	max	403.429	3	567.82	1	5.411	5	0	3	.022	1	-.005	15
504			min	-261.546	2	-489.676	3	-66.795	1	0	1	-.022	5	-.188	3
505		6	max	403.861	3	566.236	1	6.872	5	0	3	-.001	12	.117	3
506			min	-260.969	2	-490.863	3	-66.795	1	0	1	-.022	4	-.488	1
507		7	max	404.293	3	564.653	1	8.332	5	0	3	-.003	12	.422	3
508			min	-260.393	2	-492.051	3	-66.795	1	0	1	-.061	1	-.839	1
509		8	max	404.725	3	563.07	1	9.792	5	0	3	-.005	15	.727	3
510			min	-259.817	2	-493.238	3	-66.795	1	0	1	-.103	1	-1.189	1
511		9	max	415.872	3	42.094	2	50.694	5	0	9	.066	1	.849	3
512			min	-195.22	2	.475	15	-107.421	1	0	3	-.134	5	-1.353	1
513		10	max	416.304	3	40.511	2	52.154	5	0	9	0	10	.829	3
514			min	-194.643	2	-.006	5	-107.421	1	0	3	-.103	4	-1.367	1
515		11	max	416.736	3	38.928	2	53.615	5	0	9	-.004	12	.81	3
516			min	-194.067	2	-1.986	4	-107.421	1	0	3	-.083	4	-1.38	1
517		12	max	427.737	3	325.076	3	144.709	5	0	2	.101	1	.708	3
518			min	-129.422	2	-608.819	1	-64.438	1	0	3	-.226	5	-1.22	1
519		13	max	428.169	3	323.889	3	146.169	5	0	2	.061	1	.507	3
520			min	-128.846	2	-610.403	1	-64.438	1	0	3	-.135	5	-.842	1
521		14	max	428.601	3	322.701	3	147.63	5	0	2	.021	1	.306	3
522			min	-128.269	2	-611.986	1	-64.438	1	0	3	-.044	5	-.463	1
523		15	max	429.033	3	321.514	3	149.09	5	0	2	.048	5	.106	3
524			min	-127.693	2	-613.569	1	-64.438	1	0	3	-.019	1	-.082	1
525		16	max	429.466	3	320.326	3	150.55	5	0	2	.141	5	.329	2
526			min	-127.117	2	-615.152	1	-64.438	1	0	3	-.059	1	-.093	3
527		17	max	429.898	3	319.139	3	152.01	5	0	2	.235	5	.708	2
528			min	-126.541	2	-616.735	1	-64.438	1	0	3	-.099	1	-.291	3
529		18	max	24.908	5	588.533	2	-3.937	12	0	5	.196	5	.355	2
530			min	-159.784	1	-249.214	3	-106.626	4	0	2	-.144	1	-.143	3
531		19	max	25.177	5	586.95	2	-3.937	12	0	5	.14	5	.012	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-159.207	1	-250.402	3	-105.166	4	0	2	-.191	1	-.012	1
533	M5	max	353.061	1	2154.237	3	87.663	5	0	1	0	1	.03	2
534		min	10.706	12	-1647.525	1	0	1	0	4	-.193	4	0	15
535		max	353.637	1	2153.049	3	89.123	5	0	1	0	1	1.052	1
536		min	10.994	12	-1649.108	1	0	1	0	4	-.139	4	-1.331	3
537		max	1257.208	3	1595.244	1	40.845	4	0	4	0	1	2.04	1
538		min	-868.356	2	-1469.942	3	0	1	0	1	-.084	4	-2.628	3
539		max	1257.641	3	1593.661	1	42.305	4	0	4	0	1	1.051	1
540		min	-867.78	2	-1471.13	3	0	1	0	1	-.058	4	-1.715	3
541		max	1258.073	3	1592.078	1	43.765	4	0	4	0	1	.062	1
542		min	-867.204	2	-1472.317	3	0	1	0	1	-.031	4	-.802	3
543		max	1258.505	3	1590.495	1	45.225	4	0	4	0	1	.113	3
544		min	-866.628	2	-1473.505	3	0	1	0	1	-.004	5	-.925	1
545		max	1258.937	3	1588.912	1	46.685	4	0	4	.025	4	1.027	3
546		min	-866.051	2	-1474.692	3	0	1	0	1	0	1	-1.912	1
547		max	1259.369	3	1587.329	1	48.145	4	0	4	.054	4	1.943	3
548		min	-865.475	2	-1475.879	3	0	1	0	1	0	1	-2.898	1
549		max	1274.784	3	141.51	2	167.374	4	0	1	0	1	2.241	3
550		min	-728.975	2	.479	15	0	1	0	1	-.193	4	-3.29	1
551		max	1275.216	3	139.927	2	168.834	4	0	1	0	1	2.165	3
552		min	-728.399	2	.001	15	0	1	0	1	-.089	5	-3.337	1
553		max	1275.648	3	138.344	2	170.294	4	0	1	.017	4	2.09	3
554		min	-727.823	2	-1.766	6	0	1	0	1	0	1	-3.383	1
555		max	1291.354	3	945.14	3	196.877	4	0	1	0	1	1.83	3
556		min	-591.419	2	-1737.764	1	0	1	0	4	-.317	4	-3.01	1
557		max	1291.787	3	943.953	3	198.338	4	0	1	0	1	1.244	3
558		min	-590.843	2	-1739.347	1	0	1	0	4	-.195	4	-1.931	1
559		max	1292.219	3	942.766	3	199.798	4	0	1	0	1	.658	3
560		min	-590.267	2	-1740.93	1	0	1	0	4	-.071	4	-.851	1
561		max	1292.651	3	941.578	3	201.258	4	0	1	.053	4	.309	2
562		min	-589.691	2	-1742.513	1	0	1	0	4	0	1	0	15
563		max	1293.083	3	940.391	3	202.718	4	0	1	.179	4	1.384	2
564		min	-589.114	2	-1744.097	1	0	1	0	4	0	1	-.511	3
565		max	1293.515	3	939.204	3	204.178	4	0	1	.305	4	2.46	2
566		min	-588.538	2	-1745.68	1	0	1	0	4	0	1	-1.094	3
567		max	-11.545	12	1999.379	2	0	1	0	4	.3	4	1.26	2
568		min	-352.675	1	-878.416	3	-33.148	5	0	1	0	1	-.569	3
569		max	-11.256	12	1997.795	2	0	1	0	4	.281	4	.024	1
570		min	-352.099	1	-879.603	3	-31.688	5	0	1	0	1	-.023	3
571	M9	max	158.726	1	648.729	3	74.992	4	0	3	-.011	12	0	15
572		min	6.784	12	-478.854	1	3.908	12	0	4	-.188	1	-.015	2
573		max	159.302	1	647.542	3	76.452	4	0	3	-.009	12	.283	1
574		min	7.072	12	-480.437	1	3.908	12	0	4	-.147	1	-.405	3
575		max	402.564	3	570.986	1	66.795	1	0	1	-.006	12	.57	1
576		min	-262.698	2	-487.301	3	3.861	12	0	3	-.105	1	-.794	3
577		max	402.996	3	569.403	1	66.795	1	0	1	-.004	12	.217	1
578		min	-262.122	2	-488.489	3	3.861	12	0	3	-.063	1	-.491	3
579		max	403.429	3	567.82	1	66.795	1	0	1	-.001	12	-.005	15
580		min	-261.546	2	-489.676	3	3.861	12	0	3	-.029	4	-.188	3
581		max	403.861	3	566.236	1	66.795	1	0	1	.02	1	.117	3
582		min	-260.969	2	-490.863	3	3.861	12	0	3	-.016	5	-.488	1
583		max	404.293	3	564.653	1	66.795	1	0	1	.061	1	.422	3
584		min	-260.393	2	-492.051	3	3.861	12	0	3	-.006	5	-.839	1
585		max	404.725	3	563.07	1	66.795	1	0	1	.103	1	.727	3
586		min	-259.817	2	-493.238	3	3.861	12	0	3	.002	15	-1.189	1
587		max	415.872	3	42.094	2	107.421	1	0	3	-.004	12	.849	3
588		min	-195.22	2	.488	15	5.913	12	0	9	-.156	4	-1.353	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	416.304	3	40.511	2	107.421	1	0	3	.001	1	.829	3
590		min	-194.643	2	.011	15	5.913	12	0	9	-.102	4	-1.367	1
591	11	max	416.736	3	38.928	2	107.421	1	0	3	.068	1	.81	3
592		min	-194.067	2	-1.876	6	5.913	12	0	9	-.062	5	-1.38	1
593	12	max	427.737	3	325.076	3	167.673	4	0	3	-.005	12	.708	3
594		min	-129.422	2	-608.819	1	3.372	12	0	2	-.261	4	-1.22	1
595	13	max	428.169	3	323.889	3	169.133	4	0	3	-.003	12	.507	3
596		min	-128.846	2	-610.403	1	3.372	12	0	2	-.157	4	-.842	1
597	14	max	428.601	3	322.701	3	170.593	4	0	3	-.001	12	.306	3
598		min	-128.269	2	-611.986	1	3.372	12	0	2	-.051	4	-.463	1
599	15	max	429.033	3	321.514	3	172.053	4	0	3	.055	4	.106	3
600		min	-127.693	2	-613.569	1	3.372	12	0	2	0	12	-.082	1
601	16	max	429.466	3	320.326	3	173.513	4	0	3	.162	4	.329	2
602		min	-127.117	2	-615.152	1	3.372	12	0	2	.003	12	-.093	3
603	17	max	429.898	3	319.139	3	174.974	4	0	3	.271	4	.708	2
604		min	-126.541	2	-616.735	1	3.372	12	0	2	.005	12	-.291	3
605	18	max	-6.797	12	588.533	2	75.125	1	0	2	.245	4	.355	2
606		min	-159.784	1	-249.214	3	-82.026	5	0	3	.007	12	-.143	3
607	19	max	-6.509	12	586.95	2	75.125	1	0	2	.204	4	.012	3
608		min	-159.207	1	-250.402	3	-80.565	5	0	3	.01	12	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.202	1	.008	3	1.358e-2	1	NC	1	NC	1
2			min	-675	4	-.046	3	-.004	2	-2.875e-3	3	NC	1	NC	1
3		2	max	0	1	.146	3	.025	1	1.477e-2	1	NC	5	NC	2
4			min	-675	4	.004	15	-.014	5	-2.657e-3	3	1064.22	3	8333.689	1
5		3	max	0	1	.302	3	.058	1	1.597e-2	1	NC	5	NC	3
6			min	-675	4	-.01	9	-.018	5	-2.44e-3	3	587.071	3	3557.639	1
7		4	max	0	1	.398	3	.085	1	1.717e-2	1	NC	5	NC	3
8			min	-675	4	-.046	1	-.014	5	-2.222e-3	3	460.233	3	2406.43	1
9		5	max	0	1	.422	3	.098	1	1.837e-2	1	NC	5	NC	3
10			min	-675	4	-.038	1	-.005	5	-2.005e-3	3	436.209	3	2082.429	1
11		6	max	0	1	.377	3	.093	1	1.956e-2	1	NC	5	NC	3
12			min	-675	4	-.004	9	.002	10	-1.787e-3	3	483.158	3	2191.116	1
13		7	max	0	1	.276	3	.072	1	2.076e-2	1	NC	4	NC	3
14			min	-675	4	.004	15	0	10	-1.57e-3	3	635.032	3	2857.488	1
15		8	max	0	1	.241	2	.039	1	2.197e-2	2	NC	4	NC	2
16			min	-675	4	.006	15	-.005	10	-1.352e-3	3	1063.238	3	5227.129	1
17		9	max	0	1	.328	2	.024	3	2.32e-2	2	NC	4	NC	1
18			min	-675	4	.009	15	-.01	2	-1.135e-3	3	1560.766	2	NC	1
19		10	max	0	1	.37	1	.024	3	2.443e-2	2	NC	5	NC	1
20			min	-675	4	-.025	3	-.017	2	-9.173e-4	3	1204.962	2	NC	1
21		11	max	0	12	.328	2	.024	3	2.32e-2	2	NC	4	NC	1
22			min	-675	4	.009	15	-.011	5	-1.135e-3	3	1560.766	2	NC	1
23		12	max	0	12	.241	2	.039	1	2.197e-2	2	NC	4	NC	2
24			min	-675	4	.006	15	-.011	5	-1.352e-3	3	1063.238	3	5227.129	1
25		13	max	0	12	.276	3	.072	1	2.076e-2	1	NC	4	NC	3
26			min	-675	4	.003	15	-.004	5	-1.57e-3	3	635.032	3	2857.488	1
27		14	max	0	12	.377	3	.093	1	1.956e-2	1	NC	5	NC	3
28			min	-675	4	-.004	9	.002	10	-1.787e-3	3	483.158	3	2191.116	1
29		15	max	0	12	.422	3	.098	1	1.837e-2	1	NC	5	NC	3
30			min	-675	4	-.038	1	.004	10	-2.005e-3	3	436.209	3	2082.429	1
31		16	max	0	12	.398	3	.085	1	1.717e-2	1	NC	5	NC	3
32			min	-675	4	-.046	1	.003	10	-2.222e-3	3	460.233	3	2406.43	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.302	3	.058	1	1.597e-2	1	NC	5	NC	3
34		min	-.675	4	-.01	9	.002	10	-2.44e-3	3	587.071	3	3557.639	1
35	18	max	0	12	.146	3	.025	1	1.477e-2	1	NC	5	NC	2
36		min	-.675	4	.003	15	0	10	-2.657e-3	3	1064.22	3	8305.488	4
37	19	max	0	12	.202	1	.008	3	1.358e-2	1	NC	1	NC	1
38		min	-.675	4	-.046	3	-.004	2	-2.875e-3	3	NC	1	NC	1
39	M14	1	max	0	.35	3	.007	3	8.029e-3	1	NC	1	NC	1
40		min	-.515	4	-.617	1	-.004	2	-5.351e-3	3	NC	1	NC	1
41	2	max	0	1	.58	3	.016	1	9.311e-3	1	NC	5	NC	1
42		min	-.515	4	-.895	1	-.022	5	-6.313e-3	3	731.781	1	NC	1
43	3	max	0	1	.78	3	.044	1	1.059e-2	1	NC	5	NC	2
44		min	-.515	4	-1.144	1	-.027	5	-7.275e-3	3	387.091	1	4678.196	1
45	4	max	0	1	.93	3	.07	1	1.187e-2	1	NC	15	NC	3
46		min	-.515	4	-1.34	1	-.019	5	-8.237e-3	3	282.124	1	2924.29	1
47	5	max	0	1	1.021	3	.085	1	1.316e-2	1	9871.353	15	NC	3
48		min	-.515	4	-1.472	1	-.005	5	-9.199e-3	3	238.564	1	2421.492	1
49	6	max	0	1	1.049	3	.083	1	1.444e-2	1	9204.527	15	NC	3
50		min	-.515	4	-1.537	1	.002	10	-1.016e-2	3	221.515	1	2476.05	1
51	7	max	0	1	1.025	3	.065	1	1.572e-2	1	9182.682	15	NC	2
52		min	-.515	4	-1.545	1	0	10	-1.112e-2	3	219.806	1	3161.673	1
53	8	max	0	1	.967	3	.04	4	1.7e-2	1	9594.507	15	NC	2
54		min	-.515	4	-1.51	1	-.004	10	-1.209e-2	3	228.216	1	4950.564	4
55	9	max	0	1	.904	3	.027	4	1.828e-2	1	NC	15	NC	1
56		min	-.515	4	-1.462	1	-.009	2	-1.305e-2	3	241.323	1	7221.694	4
57	10	max	0	1	.872	3	.021	3	1.956e-2	1	NC	15	NC	1
58		min	-.515	4	-1.436	1	-.015	2	-1.401e-2	3	249.026	1	NC	1
59	11	max	0	12	.904	3	.022	3	1.828e-2	1	NC	15	NC	1
60		min	-.515	4	-1.462	1	-.022	5	-1.305e-2	3	241.323	1	NC	1
61	12	max	0	12	.967	3	.037	1	1.7e-2	1	9594.413	15	NC	2
62		min	-.515	4	-1.51	1	-.026	5	-1.209e-2	3	228.216	1	5669.664	1
63	13	max	0	12	1.025	3	.065	1	1.572e-2	1	9182.515	15	NC	2
64		min	-.515	4	-1.545	1	-.017	5	-1.112e-2	3	219.806	1	3161.673	1
65	14	max	0	12	1.049	3	.083	1	1.444e-2	1	9204.278	15	NC	3
66		min	-.515	4	-1.537	1	-.002	5	-1.016e-2	3	221.515	1	2476.05	1
67	15	max	0	12	1.021	3	.085	1	1.316e-2	1	9870.993	15	NC	3
68		min	-.515	4	-1.472	1	.003	10	-9.199e-3	3	238.564	1	2421.492	1
69	16	max	0	12	.93	3	.07	1	1.187e-2	1	NC	15	NC	3
70		min	-.515	4	-1.34	1	.003	10	-8.237e-3	3	282.124	1	2924.29	1
71	17	max	0	12	.78	3	.044	1	1.059e-2	1	NC	5	NC	2
72		min	-.515	4	-1.144	1	0	10	-7.275e-3	3	387.091	1	4678.196	1
73	18	max	0	12	.58	3	.028	4	9.311e-3	1	NC	5	NC	1
74		min	-.515	4	-.895	1	-.001	10	-6.313e-3	3	731.781	1	7014.429	4
75	19	max	0	12	.35	3	.007	3	8.029e-3	1	NC	1	NC	1
76		min	-.515	4	-.617	1	-.004	2	-5.351e-3	3	NC	1	NC	1
77	M15	1	max	0	.359	3	.007	3	4.493e-3	3	NC	1	NC	1
78		min	-.421	4	-.616	1	-.004	2	-8.189e-3	1	NC	1	NC	1
79	2	max	0	12	.523	3	.016	1	5.292e-3	3	NC	5	NC	1
80		min	-.421	4	-.919	1	-.032	5	-9.505e-3	1	673.659	1	6710.091	5
81	3	max	0	12	.67	3	.045	1	6.092e-3	3	NC	5	NC	2
82		min	-.421	4	-1.186	1	-.04	5	-1.082e-2	1	357.575	1	4650.381	1
83	4	max	0	12	.789	3	.071	1	6.891e-3	3	NC	15	NC	3
84		min	-.421	4	-1.394	1	-.03	5	-1.214e-2	1	262.094	1	2909.791	1
85	5	max	0	12	.873	3	.085	1	7.691e-3	3	9888.042	15	NC	3
86		min	-.421	4	-1.529	1	-.01	5	-1.345e-2	1	223.439	1	2409.751	1
87	6	max	0	12	.921	3	.084	1	8.491e-3	3	9221.971	15	NC	3
88		min	-.421	4	-1.588	1	.002	10	-1.477e-2	1	209.765	1	2462.503	1
89	7	max	0	12	.936	3	.066	1	9.29e-3	3	9202.489	15	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.421	4	-1.582	1	0	10	-1.608e-2	1	211.112	1	3138.337	1
91	8	max	0	12	.926	3	.051	4	1.009e-2	3	9618.175	15	NC	2
92		min	-.421	4	-1.531	1	-.004	10	-1.74e-2	1	222.907	1	3964.96	4
93	9	max	0	12	.905	3	.036	4	1.089e-2	3	NC	15	NC	1
94		min	-.421	4	-1.467	1	-.008	2	-1.872e-2	1	239.636	1	5543.874	4
95	10	max	0	1	.894	3	.02	3	1.169e-2	3	NC	15	NC	1
96		min	-.421	4	-1.434	1	-.014	2	-2.003e-2	1	249.328	1	NC	1
97	11	max	0	1	.905	3	.02	3	1.089e-2	3	NC	15	NC	1
98		min	-.421	4	-1.467	1	-.03	5	-1.872e-2	1	239.636	1	7068.481	5
99	12	max	0	1	.926	3	.037	1	1.009e-2	3	9618.105	15	NC	2
100		min	-.421	4	-1.531	1	-.036	5	-1.74e-2	1	222.907	1	5592.888	1
101	13	max	0	1	.936	3	.066	1	9.29e-3	3	9202.369	15	NC	2
102		min	-.421	4	-1.582	1	-.024	5	-1.608e-2	1	211.112	1	3138.337	1
103	14	max	0	1	.921	3	.084	1	8.491e-3	3	9221.795	15	NC	3
104		min	-.421	4	-1.588	1	-.003	5	-1.477e-2	1	209.765	1	2462.503	1
105	15	max	0	1	.873	3	.085	1	7.691e-3	3	9887.791	15	NC	3
106		min	-.421	4	-1.529	1	.003	10	-1.345e-2	1	223.439	1	2409.751	1
107	16	max	0	1	.789	3	.071	1	6.891e-3	3	NC	15	NC	3
108		min	-.421	4	-1.394	1	.003	10	-1.214e-2	1	262.094	1	2909.791	1
109	17	max	0	1	.67	3	.056	4	6.092e-3	3	NC	5	NC	2
110		min	-.421	4	-1.186	1	.001	10	-1.082e-2	1	357.575	1	3587.998	4
111	18	max	0	1	.523	3	.039	4	5.292e-3	3	NC	5	NC	1
112		min	-.421	4	-.919	1	-.001	10	-9.505e-3	1	673.659	1	5168.117	4
113	19	max	0	1	.359	3	.007	3	4.493e-3	3	NC	1	NC	1
114		min	-.421	4	-.616	1	-.004	2	-8.189e-3	1	NC	1	NC	1
115	M16	1	max	0	.194	1	.006	3	8.381e-3	3	NC	1	NC	1
116		min	-.138	4	-.126	3	-.003	2	-1.262e-2	1	NC	1	NC	1
117	2	max	0	12	.053	1	.025	1	9.356e-3	3	NC	5	NC	2
118		min	-.138	4	-.073	3	-.022	5	-1.36e-2	1	1346.722	2	8417.309	1
119	3	max	0	12	.004	13	.058	1	1.033e-2	3	NC	5	NC	3
120		min	-.138	4	-.094	2	-.029	5	-1.459e-2	1	753.795	2	3570.768	1
121	4	max	0	12	0	5	.085	1	1.131e-2	3	NC	5	NC	3
122		min	-.138	4	-.159	2	-.023	5	-1.557e-2	1	607.306	2	2405.546	1
123	5	max	0	12	0	13	.099	1	1.228e-2	3	NC	5	NC	3
124		min	-.138	4	-.161	2	-.01	5	-1.655e-2	1	604.384	2	2073.605	1
125	6	max	0	12	.006	4	.094	1	1.326e-2	3	NC	5	NC	3
126		min	-.138	4	-.101	2	.003	15	-1.753e-2	1	735.183	2	2170.59	1
127	7	max	0	12	.055	1	.073	1	1.423e-2	3	NC	3	NC	3
128		min	-.138	4	-.122	3	.001	10	-1.851e-2	1	1203.566	2	2804.226	1
129	8	max	0	12	.183	1	.041	1	1.521e-2	3	NC	1	NC	2
130		min	-.138	4	-.186	3	-.003	10	-1.949e-2	1	3392.938	3	4999.424	1
131	9	max	0	12	.296	1	.022	4	1.618e-2	3	NC	5	NC	1
132		min	-.138	4	-.24	3	-.006	2	-2.047e-2	1	1778.665	3	8773.554	4
133	10	max	0	1	.347	1	.017	3	1.716e-2	3	NC	5	NC	1
134		min	-.138	4	-.264	3	-.013	2	-2.145e-2	1	1338.07	1	NC	1
135	11	max	0	1	.296	1	.018	3	1.618e-2	3	NC	5	NC	1
136		min	-.138	4	-.24	3	-.017	5	-2.047e-2	1	1778.665	3	NC	1
137	12	max	0	1	.183	1	.041	1	1.521e-2	3	NC	1	NC	2
138		min	-.138	4	-.186	3	-.018	5	-1.949e-2	1	3392.938	3	4999.424	1
139	13	max	0	1	.055	1	.073	1	1.423e-2	3	NC	3	NC	3
140		min	-.138	4	-.122	3	-.008	5	-1.851e-2	1	1203.566	2	2804.226	1
141	14	max	0	1	.006	6	.094	1	1.326e-2	3	NC	5	NC	3
142		min	-.138	4	-.101	2	.004	10	-1.753e-2	1	735.183	2	2170.59	1
143	15	max	0	1	0	13	.099	1	1.228e-2	3	NC	5	NC	3
144		min	-.138	4	-.161	2	.005	10	-1.655e-2	1	604.384	2	2073.605	1
145	16	max	0	1	0	15	.085	1	1.131e-2	3	NC	5	NC	3
146		min	-.138	4	-.159	2	.005	10	-1.557e-2	1	607.306	2	2405.546	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.003	13	.058	1	1.033e-2	3	NC	5	NC	3
148			min	-1.138	4	-.094	2	.003	10	-1.459e-2	1	753.795	2	3570.768	1
149		18	max	0	1	.053	1	.031	4	9.356e-3	3	NC	5	NC	2
150			min	-1.138	4	-.073	3	0	10	-1.36e-2	1	1346.722	2	6380.836	4
151		19	max	0	1	.194	1	.006	3	8.381e-3	3	NC	1	NC	1
152			min	-1.138	4	-.126	3	-.003	2	-1.262e-2	1	NC	1	NC	1
153	M2	1	max	.007	1	.007	2	.009	1	2.417e-3	5	NC	1	NC	2
154			min	-.007	3	-.011	3	-.635	4	-1.954e-4	1	9173.939	2	95.351	4
155		2	max	.006	1	.006	2	.008	1	2.422e-3	5	NC	1	NC	2
156			min	-.007	3	-.01	3	-.583	4	-1.83e-4	1	NC	1	103.91	4
157		3	max	.006	1	.005	2	.007	1	2.426e-3	5	NC	1	NC	2
158			min	-.006	3	-.01	3	-.531	4	-1.707e-4	1	NC	1	114.09	4
159		4	max	.005	1	.004	2	.007	1	2.431e-3	5	NC	1	NC	2
160			min	-.006	3	-.01	3	-.479	4	-1.583e-4	1	NC	1	126.32	4
161		5	max	.005	1	.003	2	.006	1	2.435e-3	5	NC	1	NC	1
162			min	-.005	3	-.009	3	-.429	4	-1.46e-4	1	NC	1	141.178	4
163		6	max	.005	1	.002	2	.005	1	2.44e-3	5	NC	1	NC	1
164			min	-.005	3	-.009	3	-.38	4	-1.336e-4	1	NC	1	159.474	4
165		7	max	.004	1	.001	2	.005	1	2.446e-3	4	NC	1	NC	1
166			min	-.005	3	-.009	3	-.332	4	-1.213e-4	1	NC	1	182.356	4
167		8	max	.004	1	0	2	.004	1	2.454e-3	4	NC	1	NC	1
168			min	-.004	3	-.008	3	-.286	4	-1.089e-4	1	NC	1	211.507	4
169		9	max	.004	1	0	2	.003	1	2.461e-3	4	NC	1	NC	1
170			min	-.004	3	-.008	3	-.243	4	-9.654e-5	1	NC	1	249.47	4
171		10	max	.003	1	0	15	.003	1	2.469e-3	4	NC	1	NC	1
172			min	-.003	3	-.007	3	-.202	4	-8.419e-5	1	NC	1	300.239	4
173		11	max	.003	1	0	15	.002	1	2.477e-3	4	NC	1	NC	1
174			min	-.003	3	-.007	3	-.164	4	-7.183e-5	1	NC	1	370.386	4
175		12	max	.003	1	0	15	.002	1	2.484e-3	4	NC	1	NC	1
176			min	-.003	3	-.006	3	-.128	4	-5.948e-5	1	NC	1	471.366	4
177		13	max	.002	1	0	15	.001	1	2.492e-3	4	NC	1	NC	1
178			min	-.002	3	-.005	3	-.097	4	-4.712e-5	1	NC	1	624.637	4
179		14	max	.002	1	0	15	0	1	2.5e-3	4	NC	1	NC	1
180			min	-.002	3	-.005	3	-.069	4	-3.477e-5	1	NC	1	874.389	4
181		15	max	.001	1	0	15	0	1	2.508e-3	4	NC	1	NC	1
182			min	-.002	3	-.004	3	-.046	4	-2.241e-5	1	NC	1	1324.22	4
183		16	max	.001	1	0	15	0	1	2.515e-3	4	NC	1	NC	1
184			min	-.001	3	-.003	3	-.027	4	-1.006e-5	1	NC	1	2267.709	4
185		17	max	0	1	0	15	0	1	2.523e-3	4	NC	1	NC	1
186			min	0	3	-.002	3	-.013	4	-5.402e-7	3	NC	1	4839.095	4
187		18	max	0	1	0	15	0	1	2.531e-3	4	NC	1	NC	1
188			min	0	3	-.001	6	-.003	4	4.181e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.538e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.154e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-3.802e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-4.9e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.014	4	1.764e-4	4	NC	1	NC	1
194			min	0	2	-.002	6	0	12	8.847e-7	12	NC	1	NC	1
195		3	max	0	3	-.001	15	.028	4	8.428e-4	4	NC	1	NC	1
196			min	0	2	-.005	6	0	12	2.15e-6	12	NC	1	NC	1
197		4	max	.001	3	-.002	15	.041	4	1.509e-3	4	NC	1	NC	1
198			min	0	2	-.008	6	0	12	3.414e-6	12	NC	1	NC	1
199		5	max	.001	3	-.002	15	.054	4	2.176e-3	4	NC	1	NC	1
200			min	-.001	2	-.011	6	0	12	4.679e-6	12	9475.156	6	NC	1
201		6	max	.002	3	-.003	15	.066	4	2.842e-3	4	NC	1	NC	1
202			min	-.001	2	-.014	6	0	12	5.944e-6	12	7588.764	6	9003.683	5
203		7	max	.002	3	-.003	15	.078	4	3.508e-3	4	NC	5	NC	1



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Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.016	6	0	12	7.209e-6	12	6458.03	6	8379.088	5
205		8	max	.002	3	-.004	15	.089	4	4.175e-3	4	NC	5	NC	1
206			min	-.002	2	-.018	6	0	12	8.474e-6	12	5760.03	6	8152.618	5
207		9	max	.003	3	-.004	15	.1	4	4.841e-3	4	NC	5	NC	1
208			min	-.002	2	-.019	6	0	12	9.739e-6	12	5343.235	6	8251.013	5
209		10	max	.003	3	-.004	15	.11	4	5.508e-3	4	NC	5	NC	1
210			min	-.002	2	-.02	6	0	12	1.1e-5	12	5133.713	6	8672.753	5
211		11	max	.003	3	-.004	15	.12	4	6.174e-3	4	NC	5	NC	1
212			min	-.003	2	-.02	6	0	12	1.227e-5	12	5100.101	6	9479.782	5
213		12	max	.004	3	-.004	15	.13	4	6.84e-3	4	NC	5	NC	1
214			min	-.003	2	-.02	6	0	12	1.353e-5	12	5241.5	6	NC	1
215		13	max	.004	3	-.004	15	.139	4	7.507e-3	4	NC	5	NC	1
216			min	-.003	2	-.018	6	0	12	1.48e-5	12	5588.608	6	NC	1
217		14	max	.004	3	-.004	15	.149	4	8.173e-3	4	NC	5	NC	1
218			min	-.003	2	-.017	6	0	12	1.606e-5	12	6220.054	6	NC	1
219		15	max	.005	3	-.003	15	.158	4	8.839e-3	4	NC	2	NC	1
220			min	-.004	2	-.014	6	0	12	1.733e-5	12	7311.747	6	NC	1
221		16	max	.005	3	-.002	15	.167	4	9.506e-3	4	NC	1	NC	1
222			min	-.004	2	-.011	6	0	12	1.859e-5	12	9290.973	6	NC	1
223		17	max	.005	3	-.002	15	.177	4	1.017e-2	4	NC	1	NC	1
224			min	-.004	2	-.008	1	0	12	1.986e-5	12	NC	1	NC	1
225		18	max	.006	3	0	15	.187	4	1.084e-2	4	NC	1	NC	1
226			min	-.005	2	-.005	1	0	12	2.112e-5	12	NC	1	NC	1
227		19	max	.006	3	0	5	.198	4	1.151e-2	4	NC	1	NC	1
228			min	-.005	2	-.003	1	0	12	2.239e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.004	2	0	12	6.991e-5	1	NC	1	NC	3
230			min	0	3	-.006	3	-.198	4	-8.917e-4	5	NC	1	125.469	4
231		2	max	.003	1	.004	2	0	12	6.991e-5	1	NC	1	NC	3
232			min	0	3	-.006	3	-.182	4	-8.917e-4	5	NC	1	136.612	4
233		3	max	.003	1	.004	2	0	12	6.991e-5	1	NC	1	NC	3
234			min	0	3	-.006	3	-.166	4	-8.917e-4	5	NC	1	149.863	4
235		4	max	.002	1	.004	2	0	12	6.991e-5	1	NC	1	NC	2
236			min	0	3	-.005	3	-.15	4	-8.917e-4	5	NC	1	165.772	4
237		5	max	.002	1	.003	2	0	12	6.991e-5	1	NC	1	NC	2
238			min	0	3	-.005	3	-.134	4	-8.917e-4	5	NC	1	185.085	4
239		6	max	.002	1	.003	2	0	12	6.991e-5	1	NC	1	NC	2
240			min	0	3	-.004	3	-.119	4	-8.917e-4	5	NC	1	208.837	4
241		7	max	.002	1	.003	2	0	12	6.991e-5	1	NC	1	NC	2
242			min	0	3	-.004	3	-.104	4	-8.917e-4	5	NC	1	238.498	4
243		8	max	.002	1	.003	2	0	12	6.991e-5	1	NC	1	NC	2
244			min	0	3	-.004	3	-.09	4	-8.917e-4	5	NC	1	276.212	4
245		9	max	.002	1	.002	2	0	12	6.991e-5	1	NC	1	NC	2
246			min	0	3	-.003	3	-.076	4	-8.917e-4	5	NC	1	325.206	4
247		10	max	.001	1	.002	2	0	12	6.991e-5	1	NC	1	NC	2
248			min	0	3	-.003	3	-.064	4	-8.917e-4	5	NC	1	390.526	4
249		11	max	.001	1	.002	2	0	12	6.991e-5	1	NC	1	NC	1
250			min	0	3	-.003	3	-.052	4	-8.917e-4	5	NC	1	480.427	4
251		12	max	.001	1	.002	2	0	12	6.991e-5	1	NC	1	NC	1
252			min	0	3	-.002	3	-.041	4	-8.917e-4	5	NC	1	609.192	4
253		13	max	0	1	.001	2	0	12	6.991e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.031	4	-8.917e-4	5	NC	1	803.337	4
255		14	max	0	1	.001	2	0	12	6.991e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	-.022	4	-8.917e-4	5	NC	1	1116.816	4
257		15	max	0	1	0	2	0	12	6.991e-5	1	NC	1	NC	1
258			min	0	3	-.001	3	-.015	4	-8.917e-4	5	NC	1	1674.03	4
259		16	max	0	1	0	2	0	12	6.991e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	-.009	4	-8.917e-4	5	NC	1	2818.976	4



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	6.991e-5	1	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-8.917e-4	5	NC	1	5827.641	4
263		18	max	0	1	0	2	0	12	6.991e-5	1	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-8.917e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.991e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.917e-4	5	NC	1	NC	1
267	M6	1	max	.02	1	.024	2	0	1	2.522e-3	4	NC	3	NC	1
268			min	-.022	3	-.033	3	-.642	4	0	1	2532.789	2	94.394	4
269		2	max	.019	1	.022	2	0	1	2.524e-3	4	NC	3	NC	1
270			min	-.021	3	-.031	3	-.589	4	0	1	2775.678	2	102.868	4
271		3	max	.018	1	.02	2	0	1	2.525e-3	4	NC	3	NC	1
272			min	-.02	3	-.03	3	-.536	4	0	1	3067.863	2	112.947	4
273		4	max	.017	1	.018	2	0	1	2.526e-3	4	NC	3	NC	1
274			min	-.018	3	-.028	3	-.484	4	0	1	3423.247	2	125.055	4
275		5	max	.016	1	.016	2	0	1	2.527e-3	4	NC	3	NC	1
276			min	-.017	3	-.026	3	-.433	4	0	1	3861.161	2	139.766	4
277		6	max	.015	1	.014	2	0	1	2.529e-3	4	NC	3	NC	1
278			min	-.016	3	-.024	3	-.384	4	0	1	4409.128	2	157.88	4
279		7	max	.014	1	.012	2	0	1	2.53e-3	4	NC	3	NC	1
280			min	-.015	3	-.022	3	-.335	4	0	1	5107.436	2	180.535	4
281		8	max	.012	1	.01	2	0	1	2.531e-3	4	NC	1	NC	1
282			min	-.014	3	-.021	3	-.289	4	0	1	6017.016	2	209.397	4
283		9	max	.011	1	.008	2	0	1	2.532e-3	4	NC	1	NC	1
284			min	-.012	3	-.019	3	-.245	4	0	1	7233.667	2	246.983	4
285		10	max	.01	1	.007	2	0	1	2.534e-3	4	NC	1	NC	1
286			min	-.011	3	-.017	3	-.204	4	0	1	8915.295	2	297.249	4
287		11	max	.009	1	.005	2	0	1	2.535e-3	4	NC	1	NC	1
288			min	-.01	3	-.015	3	-.165	4	0	1	NC	1	366.702	4
289		12	max	.008	1	.004	2	0	1	2.536e-3	4	NC	1	NC	1
290			min	-.009	3	-.013	3	-.13	4	0	1	NC	1	466.685	4
291		13	max	.007	1	.003	2	0	1	2.537e-3	4	NC	1	NC	1
292			min	-.007	3	-.011	3	-.098	4	0	1	NC	1	618.444	4
293		14	max	.006	1	.002	2	0	1	2.539e-3	4	NC	1	NC	1
294			min	-.006	3	-.01	3	-.07	4	0	1	NC	1	865.736	4
295		15	max	.005	1	.001	2	0	1	2.54e-3	4	NC	1	NC	1
296			min	-.005	3	-.008	3	-.046	4	0	1	NC	1	1311.148	4
297		16	max	.003	1	0	2	0	1	2.541e-3	4	NC	1	NC	1
298			min	-.004	3	-.006	3	-.027	4	0	1	NC	1	2245.403	4
299		17	max	.002	1	0	2	0	1	2.542e-3	4	NC	1	NC	1
300			min	-.002	3	-.004	3	-.013	4	0	1	NC	1	4791.777	4
301		18	max	.001	1	0	2	0	1	2.544e-3	4	NC	1	NC	1
302			min	-.001	3	-.002	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.545e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-4.903e-4	4	NC	1	NC	1
307		2	max	.001	3	0	15	.014	4	1.576e-4	4	NC	1	NC	1
308			min	0	2	-.003	3	0	1	0	1	NC	1	NC	1
309		3	max	.002	3	-.001	15	.028	4	8.056e-4	4	NC	1	NC	1
310			min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
311		4	max	.003	3	-.002	15	.041	4	1.454e-3	4	NC	1	NC	1
312			min	-.003	2	-.009	3	0	1	0	1	NC	1	NC	1
313		5	max	.004	3	-.003	15	.054	4	2.101e-3	4	NC	1	NC	1
314			min	-.004	2	-.012	3	0	1	0	1	8832.398	3	9053.375	4
315		6	max	.005	3	-.003	15	.066	4	2.749e-3	4	NC	1	NC	1
316			min	-.005	2	-.015	3	0	1	0	1	7409.313	3	7916.439	4
317		7	max	.006	3	-.004	15	.078	4	3.397e-3	4	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.006	2	-.017	3	0	1	0	1	6481.993	4	7306.56	4
319	8	max	.007	3	-.004	15	.089	4	4.045e-3	4	NC	2	NC	1
320		min	-.007	2	-.018	4	0	1	0	1	5779.732	4	7039.257	4
321	9	max	.008	3	-.005	15	.099	4	4.693e-3	4	NC	2	NC	1
322		min	-.008	2	-.02	4	0	1	0	1	5360.238	4	7040.032	4
323	10	max	.009	3	-.005	15	.11	4	5.341e-3	4	NC	5	NC	1
324		min	-.009	2	-.021	4	0	1	0	1	5149.034	4	7293.325	4
325	11	max	.01	3	-.005	15	.119	4	5.989e-3	4	NC	5	NC	1
326		min	-.01	2	-.021	4	0	1	0	1	5114.48	4	7829.768	4
327	12	max	.011	3	-.005	15	.129	4	6.637e-3	4	NC	5	NC	1
328		min	-.011	2	-.02	4	0	1	0	1	5255.553	4	8735.091	4
329	13	max	.012	3	-.004	15	.137	4	7.285e-3	4	NC	5	NC	1
330		min	-.012	2	-.019	4	0	1	0	1	5602.948	4	NC	1
331	14	max	.013	3	-.004	15	.146	4	7.933e-3	4	NC	2	NC	1
332		min	-.012	2	-.017	4	0	1	0	1	6235.421	4	NC	1
333	15	max	.015	3	-.003	15	.155	4	8.581e-3	4	NC	1	NC	1
334		min	-.013	2	-.015	4	0	1	0	1	7329.244	4	NC	1
335	16	max	.016	3	-.003	15	.164	4	9.229e-3	4	NC	1	NC	1
336		min	-.014	2	-.012	4	0	1	0	1	9312.643	4	NC	1
337	17	max	.017	3	-.002	15	.172	4	9.877e-3	4	NC	1	NC	1
338		min	-.015	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.018	3	-.001	15	.182	4	1.052e-2	4	NC	1	NC	1
340		min	-.016	2	-.008	1	0	1	0	1	NC	1	NC	1
341	19	max	.019	3	0	15	.191	4	1.117e-2	4	NC	1	NC	1
342		min	-.017	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.016	2	0	0	1	NC	1	NC	1
344		min	-.002	3	-.019	3	-.191	4	-1.003e-3	4	NC	1	129.735	4
345	2	max	.007	1	.015	2	0	1	0	1	NC	1	NC	1
346		min	-.002	3	-.018	3	-.176	4	-1.003e-3	4	NC	1	141.269	4
347	3	max	.007	1	.014	2	0	1	0	1	NC	1	NC	1
348		min	-.002	3	-.017	3	-.16	4	-1.003e-3	4	NC	1	154.983	4
349	4	max	.007	1	.013	2	0	1	0	1	NC	1	NC	1
350		min	-.002	3	-.016	3	-.145	4	-1.003e-3	4	NC	1	171.448	4
351	5	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
352		min	-.001	3	-.015	3	-.13	4	-1.003e-3	4	NC	1	191.435	4
353	6	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
354		min	-.001	3	-.014	3	-.115	4	-1.003e-3	4	NC	1	216.016	4
355	7	max	.005	1	.011	2	0	1	0	1	NC	1	NC	1
356		min	-.001	3	-.013	3	-.101	4	-1.003e-3	4	NC	1	246.712	4
357	8	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
358		min	-.001	3	-.011	3	-.087	4	-1.003e-3	4	NC	1	285.742	4
359	9	max	.004	1	.009	2	0	1	0	1	NC	1	NC	1
360		min	-.001	3	-.01	3	-.074	4	-1.003e-3	4	NC	1	336.445	4
361	10	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.009	3	-.061	4	-1.003e-3	4	NC	1	404.043	4
363	11	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.008	3	-.05	4	-1.003e-3	4	NC	1	497.082	4
365	12	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.007	3	-.039	4	-1.003e-3	4	NC	1	630.343	4
367	13	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.006	3	-.03	4	-1.003e-3	4	NC	1	831.269	4
369	14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.005	3	-.021	4	-1.003e-3	4	NC	1	1155.703	4
371	15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.004	3	-.014	4	-1.003e-3	4	NC	1	1732.403	4
373	16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.003	3	-.009	4	-1.003e-3	4	NC	1	2917.424	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.004	4	-1.003e-3	4	NC	1	6031.524	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-1.003e-3	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-1.003e-3	4	NC	1	NC	1
381	M10	1	max	.007	1	.007	2	0	12	2.503e-3	4	NC	1	NC	2
382			min	-.007	3	-.011	3	-.64	4	1.209e-5	12	9173.939	2	94.658	4
383		2	max	.006	1	.006	2	0	12	2.504e-3	4	NC	1	NC	2
384			min	-.007	3	-.01	3	-.587	4	1.135e-5	12	NC	1	103.155	4
385		3	max	.006	1	.005	2	0	12	2.506e-3	4	NC	1	NC	2
386			min	-.006	3	-.01	3	-.535	4	1.061e-5	12	NC	1	113.263	4
387		4	max	.005	1	.004	2	0	12	2.507e-3	4	NC	1	NC	2
388			min	-.006	3	-.01	3	-.483	4	9.879e-6	12	NC	1	125.404	4
389		5	max	.005	1	.003	2	0	12	2.508e-3	4	NC	1	NC	1
390			min	-.005	3	-.009	3	-.432	4	9.144e-6	12	NC	1	140.156	4
391		6	max	.005	1	.002	2	0	12	2.509e-3	4	NC	1	NC	1
392			min	-.005	3	-.009	3	-.383	4	8.408e-6	12	NC	1	158.32	4
393		7	max	.004	1	.001	2	0	12	2.51e-3	4	NC	1	NC	1
394			min	-.005	3	-.009	3	-.335	4	7.673e-6	12	NC	1	181.038	4
395		8	max	.004	1	0	2	0	12	2.511e-3	4	NC	1	NC	1
396			min	-.004	3	-.008	3	-.288	4	6.937e-6	12	NC	1	209.981	4
397		9	max	.004	1	0	2	0	12	2.512e-3	4	NC	1	NC	1
398			min	-.004	3	-.008	3	-.245	4	6.201e-6	12	NC	1	247.672	4
399		10	max	.003	1	0	2	0	12	2.513e-3	4	NC	1	NC	1
400			min	-.003	3	-.007	3	-.203	4	5.466e-6	12	NC	1	298.078	4
401		11	max	.003	1	-.001	2	0	12	2.515e-3	4	NC	1	NC	1
402			min	-.003	3	-.007	3	-.165	4	4.73e-6	12	NC	1	367.725	4
403		12	max	.003	1	-.001	2	0	12	2.516e-3	4	NC	1	NC	1
404			min	-.003	3	-.006	3	-.129	4	3.995e-6	12	NC	1	467.988	4
405		13	max	.002	1	-.001	15	0	12	2.517e-3	4	NC	1	NC	1
406			min	-.002	3	-.005	3	-.098	4	3.259e-6	12	NC	1	620.172	4
407		14	max	.002	1	-.001	15	0	12	2.518e-3	4	NC	1	NC	1
408			min	-.002	3	-.005	3	-.07	4	2.524e-6	12	NC	1	868.16	4
409		15	max	.001	1	-.001	15	0	12	2.519e-3	4	NC	1	NC	1
410			min	-.002	3	-.004	4	-.046	4	1.668e-6	10	NC	1	1314.831	4
411		16	max	.001	1	0	15	0	12	2.52e-3	4	NC	1	NC	1
412			min	-.001	3	-.003	4	-.027	4	6.376e-7	10	NC	1	2251.748	4
413		17	max	0	1	0	15	0	12	2.521e-3	4	NC	1	NC	1
414			min	0	3	-.002	4	-.013	4	-2.296e-6	1	NC	1	4805.481	4
415		18	max	0	1	0	15	0	12	2.522e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.004	4	-1.465e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.524e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.701e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	8.276e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-4.855e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.014	4	1.675e-4	4	NC	1	NC	1
422			min	0	2	-.003	4	0	1	-1.691e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	.028	4	8.204e-4	4	NC	1	NC	1
424			min	0	2	-.006	4	0	1	-4.21e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	.041	4	1.473e-3	4	NC	1	NC	1
426			min	0	2	-.009	4	0	1	-6.73e-5	1	NC	1	NC	1
427		5	max	.001	3	-.003	15	.053	4	2.126e-3	4	NC	1	NC	1
428			min	-.001	2	-.012	4	0	1	-9.249e-5	1	9035.093	4	9831.19	4
429		6	max	.002	3	-.004	15	.066	4	2.779e-3	4	NC	1	NC	1
430			min	-.001	2	-.014	4	0	1	-1.177e-4	1	7269.882	4	8641.095	4
431		7	max	.002	3	-.004	15	.077	4	3.432e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.017	4	-.001	1	-1.429e-4	1	6209.561	4	8023.698	4
433		8	max	.002	3	-.005	15	.088	4	4.085e-3	4	NC	5	NC	1
434			min	-.002	2	-.019	4	-.001	1	-1.681e-4	1	5555.074	4	7785.403	4
435		9	max	.003	3	-.005	15	.099	4	4.738e-3	4	NC	5	NC	1
436			min	-.002	2	-.02	4	-.002	1	-1.932e-4	1	5165.876	4	7852.641	4
437		10	max	.003	3	-.005	15	.109	4	5.391e-3	4	NC	5	NC	1
438			min	-.002	2	-.021	4	-.002	1	-2.184e-4	1	4973.535	4	8219.076	4
439		11	max	.003	3	-.005	15	.119	4	6.044e-3	4	NC	5	NC	1
440			min	-.003	2	-.021	4	-.003	1	-2.436e-4	1	4949.489	4	8935.661	4
441		12	max	.004	3	-.005	15	.128	4	6.697e-3	4	NC	5	NC	1
442			min	-.003	2	-.021	4	-.003	1	-2.688e-4	1	5094.062	4	NC	1
443		13	max	.004	3	-.005	15	.137	4	7.35e-3	4	NC	5	NC	1
444			min	-.003	2	-.02	4	-.003	1	-2.94e-4	1	5437.968	4	NC	1
445		14	max	.004	3	-.004	15	.146	4	8.003e-3	4	NC	5	NC	1
446			min	-.003	2	-.018	4	-.004	1	-3.192e-4	1	6058.449	4	NC	1
447		15	max	.005	3	-.004	15	.155	4	8.656e-3	4	NC	2	NC	1
448			min	-.004	2	-.015	4	-.005	1	-3.444e-4	1	7127.572	4	NC	1
449		16	max	.005	3	-.003	15	.164	4	9.309e-3	4	NC	1	NC	1
450			min	-.004	2	-.012	4	-.005	1	-3.696e-4	1	9062.724	4	NC	1
451		17	max	.005	3	-.002	15	.173	4	9.962e-3	4	NC	1	NC	1
452			min	-.004	2	-.009	4	-.006	1	-3.948e-4	1	NC	1	NC	1
453		18	max	.006	3	-.001	15	.183	4	1.061e-2	4	NC	1	NC	1
454			min	-.005	2	-.005	1	-.007	1	-4.2e-4	1	NC	1	NC	1
455		19	max	.006	3	0	10	.193	4	1.127e-2	4	NC	1	NC	1
456			min	-.005	2	-.003	1	-.008	1	-4.452e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.004	2	.008	1	-3.958e-6	12	NC	1	NC	3
458			min	0	3	-.006	3	-.193	4	-9.266e-4	4	NC	1	128.306	4
459		2	max	.003	1	.004	2	.008	1	-3.958e-6	12	NC	1	NC	3
460			min	0	3	-.006	3	-.178	4	-9.266e-4	4	NC	1	139.704	4
461		3	max	.003	1	.004	2	.007	1	-3.958e-6	12	NC	1	NC	3
462			min	0	3	-.006	3	-.162	4	-9.266e-4	4	NC	1	153.259	4
463		4	max	.002	1	.004	2	.006	1	-3.958e-6	12	NC	1	NC	2
464			min	0	3	-.005	3	-.146	4	-9.266e-4	4	NC	1	169.532	4
465		5	max	.002	1	.003	2	.006	1	-3.958e-6	12	NC	1	NC	2
466			min	0	3	-.005	3	-.131	4	-9.266e-4	4	NC	1	189.286	4
467		6	max	.002	1	.003	2	.005	1	-3.958e-6	12	NC	1	NC	2
468			min	0	3	-.004	3	-.116	4	-9.266e-4	4	NC	1	213.582	4
469		7	max	.002	1	.003	2	.004	1	-3.958e-6	12	NC	1	NC	2
470			min	0	3	-.004	3	-.102	4	-9.266e-4	4	NC	1	243.921	4
471		8	max	.002	1	.003	2	.004	1	-3.958e-6	12	NC	1	NC	2
472			min	0	3	-.004	3	-.088	4	-9.266e-4	4	NC	1	282.498	4
473		9	max	.002	1	.002	2	.003	1	-3.958e-6	12	NC	1	NC	2
474			min	0	3	-.003	3	-.075	4	-9.266e-4	4	NC	1	332.612	4
475		10	max	.001	1	.002	2	.003	1	-3.958e-6	12	NC	1	NC	2
476			min	0	3	-.003	3	-.062	4	-9.266e-4	4	NC	1	399.426	4
477		11	max	.001	1	.002	2	.002	1	-3.958e-6	12	NC	1	NC	1
478			min	0	3	-.003	3	-.05	4	-9.266e-4	4	NC	1	491.383	4
479		12	max	.001	1	.002	2	.002	1	-3.958e-6	12	NC	1	NC	1
480			min	0	3	-.002	3	-.04	4	-9.266e-4	4	NC	1	623.094	4
481		13	max	0	1	.001	2	.001	1	-3.958e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.03	4	-9.266e-4	4	NC	1	821.681	4
483		14	max	0	1	.001	2	0	1	-3.958e-6	12	NC	1	NC	1
484			min	0	3	-.002	3	-.022	4	-9.266e-4	4	NC	1	1142.333	4
485		15	max	0	1	0	2	0	1	-3.958e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.014	4	-9.266e-4	4	NC	1	1712.303	4
487		16	max	0	1	0	2	0	1	-3.958e-6	12	NC	1	NC	1
488			min	0	3	-.001	3	-.009	4	-9.266e-4	4	NC	1	2883.47	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	-3.958e-6	12	NC	1	NC	1
490		min	0	3	0	3	-.004	4	-9.266e-4	4	NC	1	5961.074	4
491	18	max	0	1	0	2	0	1	-3.958e-6	12	NC	1	NC	1
492		min	0	3	0	3	-.001	4	-9.266e-4	4	NC	1	NC	1
493	19	max	0	1	0	1	0	1	-3.958e-6	12	NC	1	NC	1
494		min	0	1	0	1	0	1	-9.266e-4	4	NC	1	NC	1
495	M1	1	max	.008	3	.202	.675	4	1.062e-2	1	NC	1	NC	1
496		min	-.004	2	-.046	3	0	12	-1.755e-2	3	NC	1	NC	1
497	2	max	.008	3	.101	1	.656	4	8.799e-3	4	NC	5	NC	1
498		min	-.004	2	-.023	3	-.006	1	-8.71e-3	3	1323.041	1	NC	1
499	3	max	.008	3	.011	3	.635	4	1.545e-2	4	NC	5	NC	1
500		min	-.004	2	-.01	2	-.009	1	-1.853e-4	1	636.84	1	6425.339	5
501	4	max	.008	3	.065	3	.614	4	1.342e-2	4	NC	15	NC	1
502		min	-.004	2	-.133	1	-.008	1	-3.922e-3	3	401.782	1	4543.691	5
503	5	max	.008	3	.132	3	.592	4	1.139e-2	4	9835.497	15	NC	1
504		min	-.004	2	-.262	1	-.006	1	-7.751e-3	3	289.667	1	3583.39	5
505	6	max	.007	3	.205	3	.57	4	1.387e-2	1	7785.506	15	NC	1
506		min	-.004	2	-.388	1	-.003	1	-1.158e-2	3	227.921	1	3000.996	5
507	7	max	.007	3	.274	3	.548	4	1.855e-2	1	6573.898	15	NC	1
508		min	-.004	2	-.5	1	0	3	-1.541e-2	3	191.494	1	2602.28	4
509	8	max	.007	3	.332	3	.524	4	2.324e-2	1	5856.381	15	NC	1
510		min	-.004	2	-.589	1	0	12	-1.924e-2	3	169.96	1	2310.221	4
511	9	max	.007	3	.369	3	.5	4	2.553e-2	1	5480.754	15	NC	1
512		min	-.004	2	-.646	1	0	1	-1.965e-2	3	158.736	1	2122.084	4
513	10	max	.007	3	.384	3	.472	4	2.622e-2	1	5365.908	15	NC	1
514		min	-.004	2	-.664	1	0	10	-1.776e-2	3	155.373	1	2063.86	4
515	11	max	.007	3	.375	3	.44	4	2.691e-2	1	5480.55	15	NC	1
516		min	-.004	2	-.645	1	0	12	-1.588e-2	3	158.967	1	2104.784	4
517	12	max	.006	3	.343	3	.406	4	2.534e-2	1	5855.89	15	NC	1
518		min	-.004	2	-.588	1	0	1	-1.366e-2	3	170.656	1	2251.718	4
519	13	max	.006	3	.292	3	.367	4	2.038e-2	1	6572.933	15	NC	1
520		min	-.003	2	-.497	1	0	1	-1.093e-2	3	193.169	1	2674.441	4
521	14	max	.006	3	.227	3	.324	4	1.542e-2	1	7783.728	15	NC	1
522		min	-.003	2	-.383	1	0	12	-8.199e-3	3	231.46	1	3620.796	4
523	15	max	.006	3	.154	3	.28	4	1.047e-2	1	9832.235	15	NC	1
524		min	-.003	2	-.255	1	0	12	-5.47e-3	3	296.858	1	5897.393	4
525	16	max	.006	3	.078	3	.237	4	9.476e-3	4	NC	15	NC	1
526		min	-.003	2	-.126	1	0	12	-2.74e-3	3	416.685	1	NC	1
527	17	max	.006	3	.004	3	.198	4	1.061e-2	4	NC	5	NC	1
528		min	-.003	2	-.006	2	0	12	-1.103e-5	3	669.861	1	NC	1
529	18	max	.006	3	.099	1	.166	4	6.999e-3	2	NC	5	NC	1
530		min	-.003	2	-.063	3	0	12	-2.389e-3	3	1406.082	1	NC	1
531	19	max	.006	3	.194	1	.138	4	1.394e-2	2	NC	1	NC	1
532		min	-.003	2	-.126	3	0	1	-4.858e-3	3	NC	1	NC	1
533	M5	1	max	.024	3	.37	.675	4	0	1	NC	1	NC	1
534		min	-.017	2	-.025	3	0	1	-8.148e-6	4	NC	1	NC	1
535	2	max	.024	3	.186	1	.66	4	7.901e-3	4	NC	5	NC	1
536		min	-.017	2	-.014	3	0	1	0	1	730.741	1	9020.264	4
537	3	max	.024	3	.033	3	.641	4	1.562e-2	4	NC	15	NC	1
538		min	-.017	2	-.029	2	0	1	0	1	338.486	1	5234.473	4
539	4	max	.024	3	.147	3	.62	4	1.273e-2	4	7697.13	15	NC	1
540		min	-.016	2	-.293	1	0	1	0	1	203.266	1	3969.263	4
541	5	max	.023	3	.309	3	.596	4	9.835e-3	4	5353.754	15	NC	1
542		min	-.016	2	-.587	1	0	1	0	1	140.812	1	3333.606	4
543	6	max	.023	3	.492	3	.572	4	6.942e-3	4	4103.225	15	NC	1
544		min	-.016	2	-.882	1	0	1	0	1	107.568	1	2931.806	4
545	7	max	.022	3	.673	3	.547	4	4.05e-3	4	3384.321	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.015	2	-1.152	1	0	1	0	1	88.492	1	2622.042	4
547		8	max	.022	3	.825	3	.524	4	1.157e-3	4	2967.873	15	NC	1
548			min	-.015	2	-1.37	1	0	1	0	1	77.457	1	2340.922	4
549		9	max	.021	3	.923	3	.5	4	1.094e-7	14	2754.637	15	NC	1
550			min	-.015	2	-1.507	1	0	1	-4.261e-6	5	71.816	1	2117.614	4
551		10	max	.021	3	.959	3	.471	4	2.318e-7	14	2690.38	15	NC	1
552			min	-.014	2	-1.553	1	0	1	-4.04e-6	5	70.142	1	2082.876	4
553		11	max	.02	3	.936	3	.44	4	3.543e-7	14	2754.749	15	NC	1
554			min	-.014	2	-1.506	1	0	1	-3.819e-6	5	71.935	1	2134.9	4
555		12	max	.02	3	.854	3	.408	4	7.564e-4	4	2968.138	15	NC	1
556			min	-.014	2	-1.366	1	0	1	0	1	77.852	1	2210.379	4
557		13	max	.019	3	.722	3	.369	4	2.647e-3	4	3384.854	15	NC	1
558			min	-.014	2	-1.143	1	0	1	0	1	89.532	1	2604.618	4
559		14	max	.019	3	.555	3	.324	4	4.538e-3	4	4104.257	15	NC	1
560			min	-.013	2	-.866	1	0	1	0	1	109.944	1	3671.436	4
561		15	max	.018	3	.37	3	.277	4	6.428e-3	4	5355.782	15	NC	1
562			min	-.013	2	-.566	1	0	1	0	1	146.059	1	6875.267	5
563		16	max	.018	3	.184	3	.232	4	8.319e-3	4	7701.373	15	NC	1
564			min	-.013	2	-.272	1	0	1	0	1	215.285	1	NC	1
565		17	max	.017	3	.011	3	.192	4	1.021e-2	4	NC	15	NC	1
566			min	-.013	2	-.016	2	0	1	0	1	368.457	1	NC	1
567		18	max	.017	3	.183	1	.161	4	5.165e-3	4	NC	5	NC	1
568			min	-.013	2	-.135	3	0	1	0	1	812.999	1	NC	1
569		19	max	.017	3	.347	1	.138	4	0	1	NC	1	NC	1
570			min	-.013	2	-.264	3	0	1	-3.919e-6	4	NC	1	NC	1
571	M9	1	max	.008	3	.202	1	.675	4	1.755e-2	3	NC	1	NC	1
572			min	-.004	2	-.046	3	0	1	-1.062e-2	1	NC	1	NC	1
573		2	max	.008	3	.101	1	.659	4	8.71e-3	3	NC	5	NC	1
574			min	-.004	2	-.023	3	0	12	-5.129e-3	1	1323.041	1	9833.923	4
575		3	max	.008	3	.011	3	.639	4	1.556e-2	4	NC	5	NC	1
576			min	-.004	2	-.01	2	0	12	-1.497e-5	10	636.84	1	5582.376	4
577		4	max	.008	3	.065	3	.618	4	1.223e-2	5	NC	15	NC	1
578			min	-.004	2	-.133	1	0	12	-4.5e-3	1	401.782	1	4131.516	4
579		5	max	.008	3	.132	3	.595	4	9.233e-3	5	9798.366	15	NC	1
580			min	-.004	2	-.262	1	0	12	-9.185e-3	1	289.667	1	3392.908	4
581		6	max	.007	3	.205	3	.572	4	1.158e-2	3	7757.228	15	NC	1
582			min	-.004	2	-.388	1	0	12	-1.387e-2	1	227.921	1	2932.839	4
583		7	max	.007	3	.274	3	.548	4	1.541e-2	3	6550.709	15	NC	1
584			min	-.004	2	-.5	1	0	1	-1.855e-2	1	191.494	1	2599.488	4
585		8	max	.007	3	.332	3	.524	4	1.924e-2	3	5836.137	15	NC	1
586			min	-.004	2	-.589	1	0	1	-2.324e-2	1	169.96	1	2325.226	5
587		9	max	.007	3	.369	3	.5	4	1.965e-2	3	5462.013	15	NC	1
588			min	-.004	2	-.646	1	0	12	-2.553e-2	1	158.736	1	2116.237	4
589		10	max	.007	3	.384	3	.472	4	1.776e-2	3	5347.595	15	NC	1
590			min	-.004	2	-.664	1	0	1	-2.622e-2	1	155.373	1	2064.707	4
591		11	max	.007	3	.375	3	.44	4	1.588e-2	3	5461.778	15	NC	1
592			min	-.004	2	-.645	1	0	1	-2.691e-2	1	158.967	1	2111.819	4
593		12	max	.006	3	.343	3	.407	4	1.366e-2	3	5835.691	15	NC	1
594			min	-.004	2	-.588	1	0	12	-2.534e-2	1	170.656	1	2237.251	4
595		13	max	.006	3	.292	3	.367	4	1.093e-2	3	6550.024	15	NC	1
596			min	-.003	2	-.497	1	0	10	-2.038e-2	1	193.169	1	2672.874	4
597		14	max	.006	3	.227	3	.323	4	8.199e-3	3	7756.205	15	NC	1
598			min	-.003	2	-.383	1	-.002	1	-1.542e-2	1	231.46	1	3698.066	5
599		15	max	.006	3	.154	3	.277	4	6.113e-3	5	9796.792	15	NC	1
600			min	-.003	2	-.255	1	-.005	1	-1.047e-2	1	296.858	1	6355.426	5
601		16	max	.006	3	.078	3	.233	4	8.189e-3	5	NC	15	NC	1
602			min	-.003	2	-.126	1	-.008	1	-5.514e-3	1	416.685	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 3, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.006	3	.004	3	.194	4	1.033e-2	4	NC	5	NC	1
604		min	-.003	2	-.006	2	-.008	1	-5.588e-4	1	669.861	1	NC	1
605	18	max	.006	3	.099	1	.162	4	4.99e-3	5	NC	5	NC	1
606		min	-.003	2	-.063	3	-.006	1	-6.999e-3	2	1406.082	1	NC	1
607	19	max	.006	3	.194	1	.138	4	4.858e-3	3	NC	1	NC	1
608		min	-.003	2	-.126	3	0	12	-1.394e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

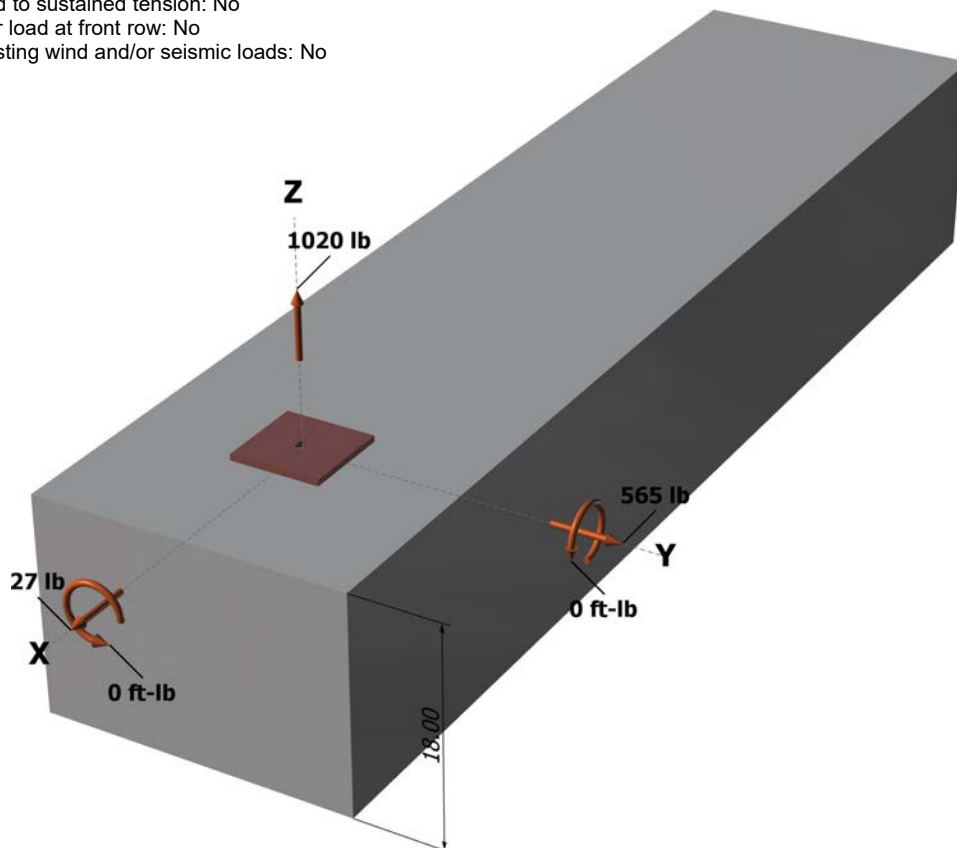
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.6025.0

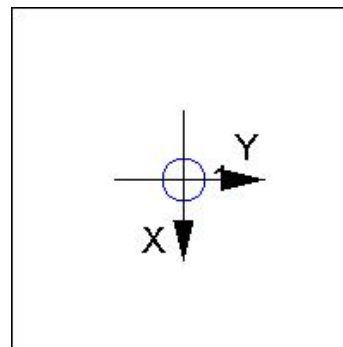
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbv}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

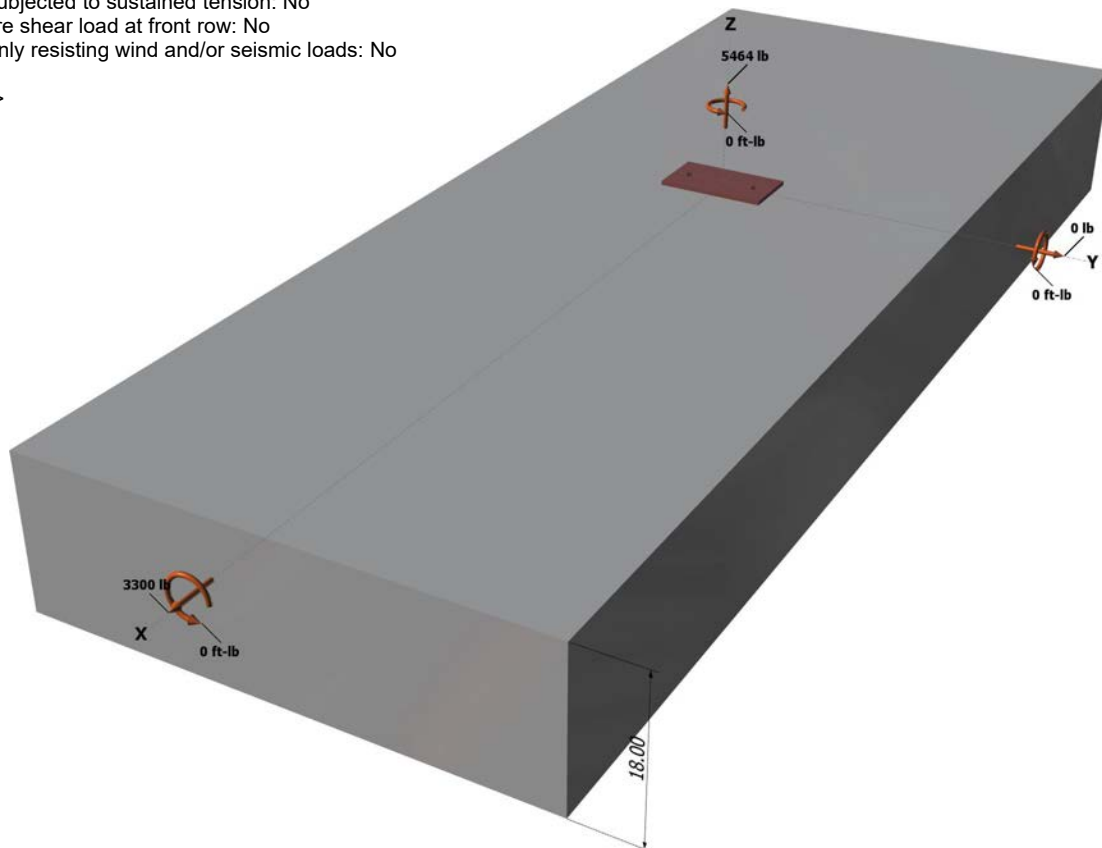
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

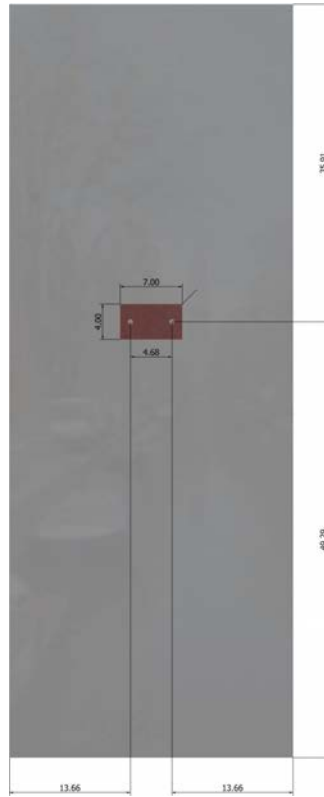
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com





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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.6025.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

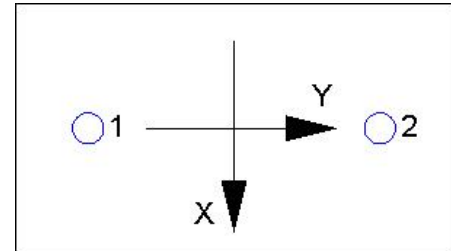
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\frac{\phi V_{cp}}{20601}$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
<b>Adhesive</b>	<b>5464</b>	<b>8093</b>	<b>0.68</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
<b>Steel</b>	<b>1650</b>	<b>3156</b>	<b>0.52</b>	<b>Pass (Governs)</b>
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.