

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	108 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-2.497 k-ft
$M_z$ =	0.008 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>91%</b>

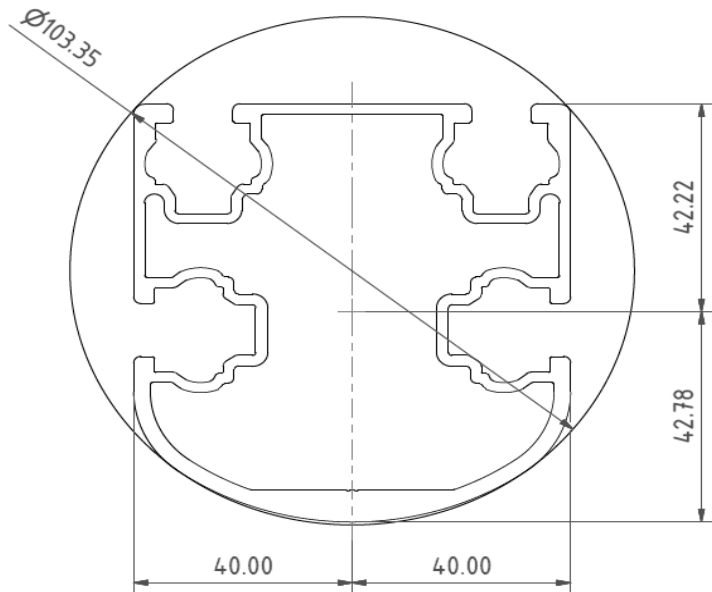


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.319 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.872 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>97%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.483 k-ft
$P_n$ =	0.562 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>36%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.477 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>34%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	63.42 in
$\Phi F_{ty \text{ AXIAL}}$ =	12.77 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.550 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	12.545 k
Utilization =	<u>29%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

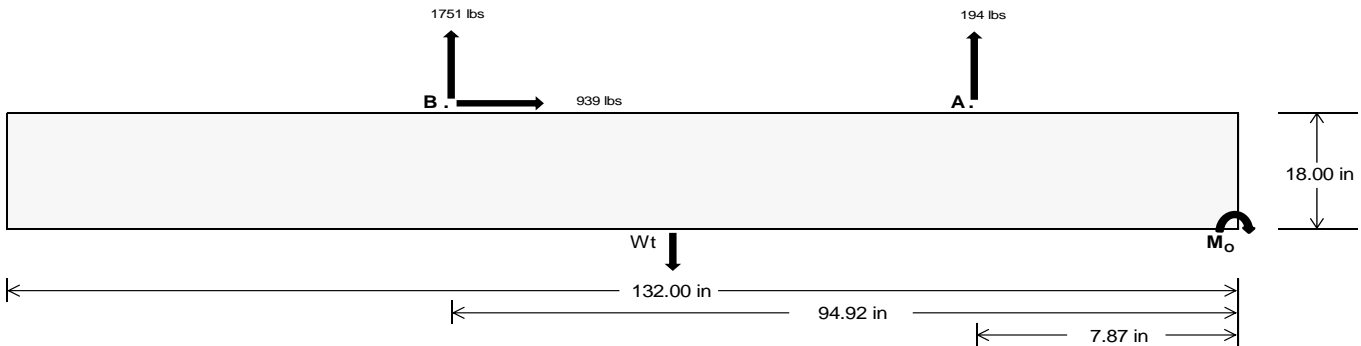
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>818.31</u>	<u>7293.56</u> k
Compressive Load =		<u>3948.07</u>	<u>5481.53</u> k
Lateral Load =		<u>329.85</u>	<u>3905.45</u> k
Moment (Weak Axis) =		<u>0.65</u>	<u>0.28</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 184621.9$  in-lbs  
Resisting Force Required = 2797.30 lbs  
S.F. = 1.67  
Weight Required = 4662.17 lbs  
Minimum Width = 39 in  
Weight Provided = 7775.63 lbs

### Sliding

Force = 938.71 lbs  
Friction = 0.4  
Weight Required = 2346.76 lbs  
Resisting Weight = 7775.63 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 938.71 lbs  
Cohesion = 130 psf  
Area = 35.75 ft<sup>2</sup>  
Resisting = 3887.81 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 132in long x 39in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 39in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 39in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(3.25 \text{ ft}) =$  7776 lbs 7975 lbs 8174 lbs 8374 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	39 in	40 in	41 in	42 in	39 in	40 in	41 in	42 in	39 in	40 in	41 in	42 in	39 in	40 in	41 in	42 in
$F_A$	1249 lbs	1249 lbs	1249 lbs	1249 lbs	1640 lbs	1640 lbs	1640 lbs	1640 lbs	2049 lbs	2049 lbs	2049 lbs	2049 lbs	-388 lbs	-388 lbs	-388 lbs	-388 lbs
$F_B$	1235 lbs	1235 lbs	1235 lbs	1235 lbs	2378 lbs	2378 lbs	2378 lbs	2378 lbs	2595 lbs	2595 lbs	2595 lbs	2595 lbs	-3502 lbs	-3502 lbs	-3502 lbs	-3502 lbs
$F_V$	163 lbs	163 lbs	163 lbs	163 lbs	1680 lbs	1680 lbs	1680 lbs	1680 lbs	1368 lbs	1368 lbs	1368 lbs	1368 lbs	-1877 lbs	-1877 lbs	-1877 lbs	-1877 lbs
$P_{total}$	10259 lbs	10459 lbs	10659 lbs	10857 lbs	11794 lbs	11993 lbs	12193 lbs	12392 lbs	12420 lbs	12619 lbs	12818 lbs	13018 lbs	776 lbs	896 lbs	1015 lbs	1135 lbs
$M$	3318 lbs-ft	3318 lbs-ft	3318 lbs-ft	3318 lbs-ft	4731 lbs-ft	4731 lbs-ft	4731 lbs-ft	4731 lbs-ft	5725 lbs-ft	5725 lbs-ft	5725 lbs-ft	5725 lbs-ft	3746 lbs-ft	3746 lbs-ft	3746 lbs-ft	3746 lbs-ft
$e$	0.32 ft	0.32 ft	0.31 ft	0.31 ft	0.40 ft	0.39 ft	0.39 ft	0.38 ft	0.46 ft	0.45 ft	0.45 ft	0.44 ft	4.83 ft	4.18 ft	3.69 ft	3.30 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	236.3 psf	235.9 psf	235.4 psf	235.0 psf	257.7 psf	256.7 psf	255.8 psf	254.8 psf	260.1 psf	259.0 psf	258.0 psf	257.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	337.6 psf	334.6 psf	331.7 psf	329.0 psf	402.1 psf	397.5 psf	393.1 psf	388.9 psf	434.8 psf	429.3 psf	424.2 psf	419.2 psf	236.8 psf	136.0 psf	109.4 psf	98.3 psf

Maximum Bearing Pressure = 435 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 39in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

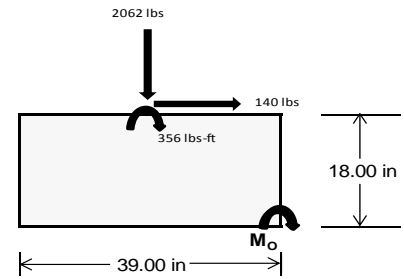
### Overturning Check

$M_o = 2784.5 \text{ ft-lbs}$   
 Resisting Force Required = 1713.54 lbs  
 S.F. = 1.67  
 Weight Required = 2855.90 lbs  
 Minimum Width = 39 in  
 Weight Provided = 7775.63 lbs

*A minimum 132in long x 39in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	39 in			39 in			39 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	262 lbs	575 lbs	190 lbs	752 lbs	2062 lbs	697 lbs	102 lbs	168 lbs	30 lbs
$F_v$	194 lbs	190 lbs	197 lbs	143 lbs	140 lbs	152 lbs	194 lbs	191 lbs	195 lbs
$P_{total}$	9889 lbs	10202 lbs	9816 lbs	9916 lbs	11226 lbs	9860 lbs	2917 lbs	2983 lbs	2845 lbs
$M$	758 lbs-ft	749 lbs-ft	766 lbs-ft	567 lbs-ft	566 lbs-ft	595 lbs-ft	758 lbs-ft	747 lbs-ft	759 lbs-ft
$e$	0.08 ft	0.07 ft	0.08 ft	0.06 ft	0.05 ft	0.06 ft	0.26 ft	0.25 ft	0.27 ft
$L/6$	0.54 ft	0.54 ft	0.54 ft	0.54 ft	0.54 ft	0.54 ft	0.54 ft	0.54 ft	0.54 ft
$f_{min}$	237.5 psf	246.7 psf	235.0 psf	248.1 psf	284.8 psf	245.1 psf	42.5 psf	44.9 psf	40.4 psf
$f_{max}$	315.7 psf	324.0 psf	314.1 psf	306.6 psf	343.2 psf	306.6 psf	120.7 psf	122.0 psf	118.8 psf



Maximum Bearing Pressure = 343 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 39in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 132in long x 39in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.854 k
Allowable Uplift =	1.214 k
Utilization =	<u>70%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.748 k
Allowable Uplift =	4.357 k
Utilization =	<u>63%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.037 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>41%</u>

#### Rear Strut

Maximum Axial Load =	4.936 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>67%</u>

#### Diagonal Strut

Maximum Axial Load =	2.594 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>35%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	46.89 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	$0.020h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.938 in
	<u><math>0.532 \leq 0.938</math>, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 108 \text{ in}$$

$$J = 0.432$$

$$298.779$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 108$$

$$J = 0.432$$

$$190.005$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.9$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \text{ in} \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 63.42 \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.46712$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7854$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 12.7711 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 12.77 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 13.14 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.9	-46.9	0	0
2	M14	Y	-46.9	-46.9	0	0
3	M15	Y	-46.9	-46.9	0	0
4	M16	Y	-46.9	-46.9	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-69.356	-69.356	0	0
2	M14	y	-69.356	-69.356	0	0
3	M15	y	-107.187	-107.187	0	0
4	M16	y	-107.187	-107.187	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	157.628	157.628	0	0
2	M14	y	119.797	119.797	0	0
3	M15	y	63.051	63.051	0	0
4	M16	y	63.051	63.051	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 25° 120mph 30psf 9ft 7-05.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	85.895	1	1390.538	3	172.618	1	.003	14	.271	1	1.515	2
20		min	5.476	12	-811.603	2	-103.595	14	-.015	2	.006	12	-2.497	3
21	11	max	85.895	1	668.646	2	-4.488	12	.015	2	.117	1	.775	2
22		min	5.476	12	-1143.19	3	-135.9	1	0	15	0	3	-1.23	3
23	12	max	85.895	1	525.689	2	-2.978	12	.015	2	.049	4	.177	2
24		min	5.476	12	-895.842	3	-99.181	1	0	15	-.005	3	-.211	3
25	13	max	85.895	1	382.732	2	-1.468	12	.015	2	.022	5	.561	3
26		min	5.476	12	-648.494	3	-62.462	1	0	15	-.081	1	-.277	2
27	14	max	85.895	1	239.775	2	.168	3	.015	2	0	15	1.086	3
28		min	4.773	15	-401.145	3	-28.388	4	0	15	-.126	1	-.588	2
29	15	max	85.895	1	96.818	2	10.976	1	.015	2	-.005	12	1.364	3
30		min	-3.002	5	-153.797	3	-20.201	5	0	15	-.133	1	-.756	2
31	16	max	85.895	1	93.551	3	47.695	1	.015	2	-.003	12	1.394	3
32		min	-13.268	5	-46.139	2	-17.865	5	0	15	-.104	1	-.782	2
33	17	max	85.895	1	340.899	3	84.413	1	.015	2	.002	3	1.176	3
34		min	-23.533	5	-189.096	2	-15.529	5	0	15	-.067	4	-.664	2
35	18	max	85.895	1	588.247	3	121.132	1	.015	2	.065	1	.712	3
36		min	-33.798	5	-332.053	2	-13.193	5	0	15	-.073	5	-.404	2
37	19	max	85.895	1	835.595	3	157.851	1	.015	2	.205	1	0	2
38		min	-44.063	5	-475.01	2	-10.857	5	0	15	-.085	5	0	3
39	M14	1	max	50.198	4	515.503	2	-7.806	12	.011	.237	1	0	1
40		min	2.37	12	-655.142	3	-163.184	1	-.013	2	.015	12	0	3
41	2	max	42.574	1	372.546	2	-6.297	12	.011	3	.143	4	.562	3
42		min	2.37	12	-468.324	3	-126.465	1	-.013	2	.008	12	-.444	2
43	3	max	42.574	1	229.589	2	-4.787	12	.011	3	.081	5	.937	3
44		min	2.37	12	-281.505	3	-89.747	1	-.013	2	-.016	1	-.745	2
45	4	max	42.574	1	86.632	2	-3.277	12	.011	3	.044	5	1.125	3
46		min	2.37	12	-94.687	3	-53.028	1	-.013	2	-.088	1	-.903	2
47	5	max	42.574	1	92.132	3	-1.108	10	.011	3	.01	5	1.126	3
48		min	-.264	5	-56.325	2	-37.596	4	-.013	2	-.122	1	-.918	2
49	6	max	42.574	1	278.95	3	20.41	1	.011	3	-.006	12	.94	3
50		min	-10.53	5	-199.282	2	-30.989	5	-.013	2	-.12	1	-.791	2
51	7	max	42.574	1	465.769	3	57.129	1	.011	3	-.005	12	.568	3
52		min	-20.795	5	-342.239	2	-28.653	5	-.013	2	-.082	1	-.52	2
53	8	max	42.574	1	652.587	3	93.848	1	.011	3	.001	10	.009	3
54		min	-31.06	5	-485.196	2	-26.317	5	-.013	2	-.083	4	-.106	2
55	9	max	42.574	1	839.406	3	130.566	1	.011	3	.106	1	.451	2
56		min	-41.325	5	-628.153	2	-23.981	5	-.013	2	-.105	5	-.737	3
57	10	max	66.224	4	1026.224	3	167.285	1	.011	3	.255	1	1.15	2
58		min	2.37	12	-771.11	2	-106.636	14	-.013	2	.006	12	-1.67	3
59	11	max	55.958	4	628.153	2	-4.272	12	.013	2	.143	4	.451	2
60		min	2.37	12	-839.406	3	-130.566	1	-.011	3	0	3	-.737	3
61	12	max	45.693	4	485.196	2	-2.762	12	.013	2	.079	5	.009	3
62		min	2.37	12	-652.587	3	-93.848	1	-.011	3	-.006	1	-.106	2
63	13	max	42.574	1	342.239	2	-1.252	12	.013	2	.042	5	.568	3
64		min	2.37	12	-465.769	3	-57.129	1	-.011	3	-.082	1	-.52	2
65	14	max	42.574	1	199.282	2	.491	3	.013	2	.007	5	.94	3
66		min	2.37	12	-278.95	3	-38.386	4	-.011	3	-.12	1	-.791	2
67	15	max	42.574	1	56.325	2	16.309	1	.013	2	-.005	12	1.126	3
68		min	2.37	12	-92.132	3	-31.167	5	-.011	3	-.122	1	-.918	2
69	16	max	42.574	1	94.687	3	53.028	1	.013	2	-.002	12	1.125	3
70		min	-4.858	5	-86.632	2	-28.831	5	-.011	3	-.088	1	-.903	2
71	17	max	42.574	1	281.505	3	89.747	1	.013	2	.003	3	.937	3
72		min	-15.123	5	-229.589	2	-26.495	5	-.011	3	-.087	4	-.745	2
73	18	max	42.574	1	468.324	3	126.465	1	.013	2	.092	1	.562	3
74		min	-25.388	5	-372.546	2	-24.159	5	-.011	3	-.108	5	-.444	2
75	19	max	42.574	1	655.142	3	163.184	1	.013	2	.237	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-35.653	5	-515.503	2	-21.824	5	-.011	3	-.131	5	0	3
77	M15	1	max	76.633	5	719.836	2	-7.722	12	.013	2	.263	4	0	2
78			min	-44.479	1	-349.076	3	-163.178	1	-.009	3	.014	12	0	3
79		2	max	66.368	5	516.349	2	-6.212	12	.013	2	.179	4	.301	3
80			min	-44.479	1	-253.051	3	-126.459	1	-.009	3	.007	12	-.618	2
81		3	max	56.103	5	312.863	2	-4.703	12	.013	2	.107	5	.506	3
82			min	-44.479	1	-157.026	3	-89.74	1	-.009	3	-.016	1	-1.033	2
83		4	max	45.837	5	109.376	2	-3.193	12	.013	2	.06	5	.615	3
84			min	-44.479	1	-61.001	3	-58.232	4	-.009	3	-.088	1	-1.244	2
85		5	max	35.572	5	35.023	3	-1.148	10	.013	2	.015	5	.628	3
86			min	-44.479	1	-94.111	2	-47.844	4	-.009	3	-.122	1	-1.251	2
87		6	max	25.307	5	131.048	3	20.417	1	.013	2	-.006	12	.545	3
88			min	-44.479	1	-297.597	2	-41.211	5	-.009	3	-.12	1	-1.056	2
89		7	max	15.042	5	227.073	3	57.135	1	.013	2	-.005	12	.366	3
90			min	-44.479	1	-501.084	2	-38.875	5	-.009	3	-.086	4	-.656	2
91		8	max	4.777	5	323.098	3	93.854	1	.013	2	.001	10	.091	3
92			min	-44.479	1	-704.57	2	-36.539	5	-.009	3	-.108	4	-.058	1
93		9	max	-2.937	12	419.123	3	130.573	1	.013	2	.106	1	.753	2
94			min	-44.479	1	-908.057	2	-34.203	5	-.009	3	-.14	5	-.28	3
95		10	max	-2.937	12	515.148	3	167.292	1	.013	2	.263	4	1.763	2
96			min	-44.479	1	-1111.543	2	-111.856	14	-.009	3	.006	12	-.747	3
97		11	max	-.262	15	908.057	2	-4.356	12	.009	3	.178	4	.753	2
98			min	-44.479	1	-419.123	3	-130.573	1	-.013	2	0	12	-.28	3
99		12	max	-2.937	12	704.57	2	-2.846	12	.009	3	.104	5	.091	3
100			min	-44.479	1	-323.098	3	-93.854	1	-.013	2	-.006	1	-.058	1
101		13	max	-2.937	12	501.084	2	-1.337	12	.009	3	.056	5	.366	3
102			min	-44.479	1	-227.073	3	-59.043	4	-.013	2	-.082	1	-.656	2
103		14	max	-2.937	12	297.597	2	.357	3	.009	3	.012	5	.545	3
104			min	-44.479	1	-131.048	3	-48.655	4	-.013	2	-.12	1	-1.056	2
105		15	max	-2.937	12	94.111	2	16.302	1	.009	3	-.005	12	.628	3
106			min	-52.073	4	-35.023	3	-41.391	5	-.013	2	-.122	1	-1.251	2
107		16	max	-2.937	12	61.001	3	53.021	1	.009	3	-.002	12	.615	3
108			min	-62.338	4	-109.376	2	-39.056	5	-.013	2	-.091	4	-1.244	2
109		17	max	-2.937	12	157.026	3	89.74	1	.009	3	.003	3	.506	3
110			min	-72.603	4	-312.863	2	-36.72	5	-.013	2	-.114	4	-1.033	2
111		18	max	-2.937	12	253.051	3	126.459	1	.009	3	.092	1	.301	3
112			min	-82.869	4	-516.349	2	-34.384	5	-.013	2	-.145	5	-.618	2
113		19	max	-2.937	12	349.076	3	163.178	1	.009	3	.236	1	0	2
114			min	-93.134	4	-719.836	2	-32.048	5	-.013	2	-.178	5	0	5
115	M16	1	max	74.922	5	680.818	2	-7.307	12	.011	2	.206	1	0	2
116			min	-91.951	1	-317.33	3	-158.146	1	-.013	3	.012	12	0	3
117		2	max	64.657	5	477.331	2	-5.797	12	.011	2	.134	4	.269	3
118			min	-91.951	1	-221.305	3	-121.427	1	-.013	3	.005	12	-.579	2
119		3	max	54.392	5	273.845	2	-4.287	12	.011	2	.08	5	.443	3
120			min	-91.951	1	-125.28	3	-84.708	1	-.013	3	-.037	1	-.955	2
121		4	max	44.127	5	70.358	2	-2.777	12	.011	2	.045	5	.52	3
122			min	-91.951	1	-29.255	3	-47.989	1	-.013	3	-.103	1	-1.127	2
123		5	max	33.861	5	66.77	3	-.698	10	.011	2	.012	5	.501	3
124			min	-91.951	1	-133.129	2	-34.458	4	-.013	3	-.133	1	-1.095	2
125		6	max	23.596	5	162.795	3	25.448	1	.011	2	-.006	12	.386	3
126			min	-91.951	1	-336.615	2	-29.109	5	-.013	3	-.126	1	-.86	2
127		7	max	13.331	5	258.82	3	62.167	1	.011	2	-.005	12	.176	3
128			min	-91.951	1	-540.102	2	-26.773	5	-.013	3	-.082	1	-.422	2
129		8	max	3.066	5	354.845	3	98.886	1	.011	2	.002	2	.22	2
130			min	-91.951	1	-743.588	2	-24.437	5	-.013	3	-.073	4	-.131	3
131		9	max	-4.737	15	450.87	3	135.605	1	.011	2	.116	1	1.065	2
132			min	-91.951	1	-947.075	2	-22.101	5	-.013	3	-.095	5	-.534	3



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-5.374	12	546.895	3	172.324	1	.011	2	.27	1	2.114	2
134		min	-91.951	1	-1150.562	2	-108.515	14	-.013	3	.007	12	-1.033	3
135	11	max	-2.92	15	947.075	2	-4.772	12	.013	3	.136	4	1.065	2
136		min	-91.951	1	-450.87	3	-135.605	1	-.011	2	.002	12	-.534	3
137	12	max	-5.374	12	743.588	2	-3.262	12	.013	3	.072	4	.22	2
138		min	-91.951	1	-354.845	3	-98.886	1	-.011	2	-.004	3	-.131	3
139	13	max	-5.374	12	540.102	2	-1.752	12	.013	3	.035	5	.176	3
140		min	-91.951	1	-258.82	3	-62.167	1	-.011	2	-.082	1	-.422	2
141	14	max	-5.374	12	336.615	2	-.242	12	.013	3	.002	5	.386	3
142		min	-91.951	1	-162.795	3	-38.183	4	-.011	2	-.126	1	-.86	2
143	15	max	-5.374	12	133.129	2	11.27	1	.013	3	-.005	12	.501	3
144		min	-91.951	1	-66.77	3	-29.951	5	-.011	2	-.133	1	-1.095	2
145	16	max	-5.374	12	29.255	3	47.989	1	.013	3	-.003	12	.52	3
146		min	-91.951	1	-70.358	2	-27.615	5	-.011	2	-.103	1	-1.127	2
147	17	max	-5.374	12	125.28	3	84.708	1	.013	3	0	3	.443	3
148		min	-91.951	1	-273.845	2	-25.279	5	-.011	2	-.093	4	-.955	2
149	18	max	-5.374	12	221.305	3	121.427	1	.013	3	.066	1	.269	3
150		min	-97.584	4	-477.331	2	-22.944	5	-.011	2	-.108	5	-.579	2
151	19	max	-5.374	12	317.33	3	158.146	1	.013	3	.206	1	0	2
152		min	-107.849	4	-680.818	2	-20.608	5	-.011	2	-.13	5	0	5
153	M2	1	max	1112.776	2	1.96	4	.555	1	0	0	3	0	1
154		min	-1522.489	3	.478	15	-38.24	4	0	4	0	2	0	1
155	2	max	1113.204	2	1.903	4	.555	1	0	3	0	1	0	15
156		min	-1522.167	3	.464	15	-38.613	4	0	4	-.011	4	0	4
157	3	max	1113.633	2	1.847	4	.555	1	0	3	0	1	0	15
158		min	-1521.846	3	.451	15	-38.986	4	0	4	-.022	4	-.001	4
159	4	max	1114.061	2	1.79	4	.555	1	0	3	0	1	0	15
160		min	-1521.524	3	.438	15	-39.36	4	0	4	-.034	4	-.002	4
161	5	max	1114.489	2	1.733	4	.555	1	0	3	0	1	0	15
162		min	-1521.203	3	.42	12	-39.733	4	0	4	-.045	4	-.002	4
163	6	max	1114.918	2	1.676	4	.555	1	0	3	0	1	0	15
164		min	-1520.882	3	.398	12	-40.106	4	0	4	-.057	4	-.003	4
165	7	max	1115.346	2	1.62	4	.555	1	0	3	0	1	0	15
166		min	-1520.56	3	.376	12	-40.48	4	0	4	-.069	4	-.003	4
167	8	max	1115.775	2	1.563	4	.555	1	0	3	.001	1	0	15
168		min	-1520.239	3	.354	12	-40.853	4	0	4	-.08	4	-.004	4
169	9	max	1116.203	2	1.506	4	.555	1	0	3	.001	1	0	12
170		min	-1519.918	3	.332	12	-41.226	4	0	4	-.092	4	-.004	4
171	10	max	1116.632	2	1.449	4	.555	1	0	3	.001	1	-.001	12
172		min	-1519.596	3	.31	12	-41.6	4	0	4	-.104	4	-.004	4
173	11	max	1117.06	2	1.392	4	.555	1	0	3	.002	1	-.001	12
174		min	-1519.275	3	.288	12	-41.973	4	0	4	-.116	4	-.005	4
175	12	max	1117.489	2	1.336	4	.555	1	0	3	.002	1	-.001	12
176		min	-1518.954	3	.265	12	-42.346	4	0	4	-.129	4	-.005	4
177	13	max	1117.917	2	1.279	4	.555	1	0	3	.002	1	-.001	12
178		min	-1518.632	3	.243	12	-42.72	4	0	4	-.141	4	-.006	4
179	14	max	1118.346	2	1.222	4	.555	1	0	3	.002	1	-.001	12
180		min	-1518.311	3	.221	12	-43.093	4	0	4	-.154	4	-.006	4
181	15	max	1118.774	2	1.165	4	.555	1	0	3	.002	1	-.001	12
182		min	-1517.989	3	.199	12	-43.466	4	0	4	-.166	4	-.006	4
183	16	max	1119.203	2	1.118	2	.555	1	0	3	.002	1	-.001	12
184		min	-1517.668	3	.177	12	-43.84	4	0	4	-.179	4	-.007	4
185	17	max	1119.631	2	1.074	2	.555	1	0	3	.003	1	-.002	12
186		min	-1517.347	3	.155	12	-44.213	4	0	4	-.192	4	-.007	4
187	18	max	1120.06	2	1.029	2	.555	1	0	3	.003	1	-.002	12
188		min	-1517.025	3	.133	12	-44.586	4	0	4	-.204	4	-.007	4
189	19	max	1120.488	2	.985	2	.555	1	0	3	.003	1	-.002	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1516.704	3	.111	12	-44.96	4	0	4	-.217	4	-.008	4
191	M3	1	max	690.003	2	7.909	4	3.199	4	0	3	0	.008	4
192		min	-827.835	3	1.871	15	.008	12	0	4	-.023	4	.002	12
193		2	max	689.833	2	7.142	4	3.738	4	0	3	0	.005	2
194		min	-827.963	3	1.691	15	.008	12	0	4	-.022	4	0	12
195		3	max	689.662	2	6.375	4	4.276	4	0	3	0	.002	2
196		min	-828.091	3	1.51	15	.008	12	0	4	-.02	4	0	3
197		4	max	689.492	2	5.608	4	4.815	4	0	3	0	0	2
198		min	-828.219	3	1.33	15	.008	12	0	4	-.018	4	-.002	3
199		5	max	689.321	2	4.84	4	5.354	4	0	3	0	0	15
200		min	-828.347	3	1.149	15	.008	12	0	4	-.016	4	-.003	3
201		6	max	689.151	2	4.073	4	5.893	4	0	3	0	1	15
202		min	-828.474	3	.969	15	.008	12	0	4	-.014	5	-.005	6
203		7	max	688.981	2	3.306	4	6.431	4	0	3	0	1	15
204		min	-828.602	3	.789	15	.008	12	0	4	-.011	5	-.007	6
205		8	max	688.81	2	2.539	4	6.97	4	0	3	0	1	15
206		min	-828.73	3	.608	15	.008	12	0	4	-.008	5	-.008	6
207		9	max	688.64	2	1.771	4	7.509	4	0	3	0	1	15
208		min	-828.858	3	.428	15	.008	12	0	4	-.005	5	-.009	6
209		10	max	688.47	2	1.004	4	8.048	4	0	3	0	1	15
210		min	-828.985	3	.215	12	.008	12	0	4	-.002	5	-.009	6
211		11	max	688.299	2	.361	2	8.586	4	0	3	.001	4	15
212		min	-829.113	3	-.143	3	.008	12	0	4	0	12	-.009	6
213		12	max	688.129	2	-.113	15	9.125	4	0	3	.005	4	15
214		min	-829.241	3	-.592	3	.008	12	0	4	0	12	-.009	6
215		13	max	687.959	2	-.293	15	9.664	4	0	3	.009	4	15
216		min	-829.369	3	-1.298	6	.008	12	0	4	0	12	-.009	6
217		14	max	687.788	2	-.474	15	10.203	4	0	3	.013	4	15
218		min	-829.496	3	-2.066	6	.008	12	0	4	0	12	-.008	6
219		15	max	687.618	2	-.654	15	10.741	4	0	3	.018	4	15
220		min	-829.624	3	-2.833	6	.008	12	0	4	0	12	-.007	6
221		16	max	687.448	2	-.834	15	11.28	4	0	3	.022	4	15
222		min	-829.752	3	-3.6	6	.008	12	0	4	0	12	-.006	6
223		17	max	687.277	2	-1.015	15	11.819	4	0	3	.027	4	15
224		min	-829.88	3	-4.367	6	.008	12	0	4	0	12	-.004	6
225		18	max	687.107	2	-1.195	15	12.358	4	0	3	.032	4	15
226		min	-830.007	3	-5.135	6	.008	12	0	4	0	12	-.002	6
227		19	max	686.937	2	-1.375	15	12.896	4	0	3	.037	4	1
228		min	-830.135	3	-5.902	6	.008	12	0	4	0	12	0	1
229	M4	1	max	1102.578	1	0	1	-.508	12	0	1	.027	4	1
230		min	-170.707	3	0	1	-252.165	4	0	1	0	12	0	1
231		2	max	1102.748	1	0	1	-.508	12	0	1	0	3	1
232		min	-170.579	3	0	1	-252.313	4	0	1	-.002	4	0	1
233		3	max	1102.918	1	0	1	-.508	12	0	1	0	12	1
234		min	-170.452	3	0	1	-252.461	4	0	1	-.031	4	0	1
235		4	max	1103.089	1	0	1	-.508	12	0	1	0	12	1
236		min	-170.324	3	0	1	-252.608	4	0	1	-.06	4	0	1
237		5	max	1103.259	1	0	1	-.508	12	0	1	0	12	1
238		min	-170.196	3	0	1	-252.756	4	0	1	-.089	4	0	1
239		6	max	1103.429	1	0	1	-.508	12	0	1	0	12	1
240		min	-170.068	3	0	1	-252.904	4	0	1	-.118	4	0	1
241		7	max	1103.6	1	0	1	-.508	12	0	1	0	12	1
242		min	-169.941	3	0	1	-253.051	4	0	1	-.147	4	0	1
243		8	max	1103.77	1	0	1	-.508	12	0	1	0	12	1
244		min	-169.813	3	0	1	-253.199	4	0	1	-.176	4	0	1
245		9	max	1103.94	1	0	1	-.508	12	0	1	0	12	1
246		min	-169.685	3	0	1	-253.346	4	0	1	-.205	4	0	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1104.111	1	0	1	-508	12	0	1	0	12	0	1
248			min	-169.557	3	0	1	-253.494	4	0	1	-.234	4	0	1
249		11	max	1104.281	1	0	1	-508	12	0	1	0	12	0	1
250			min	-169.43	3	0	1	-253.642	4	0	1	-.263	4	0	1
251		12	max	1104.451	1	0	1	-508	12	0	1	0	12	0	1
252			min	-169.302	3	0	1	-253.789	4	0	1	-.292	4	0	1
253		13	max	1104.622	1	0	1	-508	12	0	1	0	12	0	1
254			min	-169.174	3	0	1	-253.937	4	0	1	-.322	4	0	1
255		14	max	1104.792	1	0	1	-508	12	0	1	0	12	0	1
256			min	-169.046	3	0	1	-254.085	4	0	1	-.351	4	0	1
257		15	max	1104.962	1	0	1	-508	12	0	1	0	12	0	1
258			min	-168.919	3	0	1	-254.232	4	0	1	-.38	4	0	1
259		16	max	1105.133	1	0	1	-508	12	0	1	0	12	0	1
260			min	-168.791	3	0	1	-254.38	4	0	1	-.409	4	0	1
261		17	max	1105.303	1	0	1	-508	12	0	1	0	12	0	1
262			min	-168.663	3	0	1	-254.528	4	0	1	-.438	4	0	1
263		18	max	1105.473	1	0	1	-508	12	0	1	0	12	0	1
264			min	-168.535	3	0	1	-254.675	4	0	1	-.468	4	0	1
265		19	max	1105.644	1	0	1	-508	12	0	1	0	12	0	1
266			min	-168.407	3	0	1	-254.823	4	0	1	-.497	4	0	1
267	M6	1	max	3542.399	2	2.479	2	0	1	0	1	0	4	0	1
268			min	-4936.127	3	-.102	3	-38.621	4	0	4	0	1	0	1
269		2	max	3542.828	2	2.434	2	0	1	0	1	0	1	0	3
270			min	-4935.806	3	-.135	3	-38.994	4	0	4	-.011	4	0	2
271		3	max	3543.256	2	2.39	2	0	1	0	1	0	1	0	3
272			min	-4935.484	3	-.168	3	-39.368	4	0	4	-.023	4	-.001	2
273		4	max	3543.685	2	2.346	2	0	1	0	1	0	1	0	3
274			min	-4935.163	3	-.202	3	-39.741	4	0	4	-.034	4	-.002	2
275		5	max	3544.113	2	2.302	2	0	1	0	1	0	1	0	3
276			min	-4934.841	3	-.235	3	-40.114	4	0	4	-.046	4	-.003	2
277		6	max	3544.541	2	2.257	2	0	1	0	1	0	1	0	3
278			min	-4934.52	3	-.268	3	-40.488	4	0	4	-.057	4	-.003	2
279		7	max	3544.97	2	2.213	2	0	1	0	1	0	1	0	3
280			min	-4934.199	3	-.301	3	-40.861	4	0	4	-.069	4	-.004	2
281		8	max	3545.398	2	2.169	2	0	1	0	1	0	1	0	3
282			min	-4933.877	3	-.334	3	-41.234	4	0	4	-.081	4	-.005	2
283		9	max	3545.827	2	2.125	2	0	1	0	1	0	1	0	3
284			min	-4933.556	3	-.367	3	-41.608	4	0	4	-.093	4	-.005	2
285		10	max	3546.255	2	2.08	2	0	1	0	1	0	1	0	3
286			min	-4933.235	3	-.401	3	-41.981	4	0	4	-.105	4	-.006	2
287		11	max	3546.684	2	2.036	2	0	1	0	1	0	1	0	3
288			min	-4932.913	3	-.434	3	-42.354	4	0	4	-.118	4	-.007	2
289		12	max	3547.112	2	1.992	2	0	1	0	1	0	1	0	3
290			min	-4932.592	3	-.467	3	-42.728	4	0	4	-.13	4	-.007	2
291		13	max	3547.541	2	1.948	2	0	1	0	1	0	1	.001	3
292			min	-4932.271	3	-.5	3	-43.101	4	0	4	-.142	4	-.008	2
293		14	max	3547.969	2	1.903	2	0	1	0	1	0	1	.001	3
294			min	-4931.949	3	-.533	3	-43.474	4	0	4	-.155	4	-.008	2
295		15	max	3548.398	2	1.859	2	0	1	0	1	0	1	.001	3
296			min	-4931.628	3	-.567	3	-43.848	4	0	4	-.168	4	-.009	2
297		16	max	3548.826	2	1.815	2	0	1	0	1	0	1	.002	3
298			min	-4931.306	3	-.6	3	-44.221	4	0	4	-.18	4	-.009	2
299		17	max	3549.255	2	1.771	2	0	1	0	1	0	1	.002	3
300			min	-4930.985	3	-.633	3	-44.594	4	0	4	-.193	4	-.01	2
301		18	max	3549.683	2	1.726	2	0	1	0	1	0	1	.002	3
302			min	-4930.664	3	-.666	3	-44.968	4	0	4	-.206	4	-.01	2
303		19	max	3550.112	2	1.682	2	0	1	0	1	0	1	.002	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4930.342	3	-.699	3	-45.341	4	0	4	-.219	4	-.011	2
305	M7	1	max	2476.792	2	7.914	6	2.998	4	0	1	0	1	.011	2
306			min	-2592.067	3	1.858	15	0	1	0	4	-.023	4	-.002	3
307		2	max	2476.621	2	7.147	6	3.537	4	0	1	0	1	.008	2
308			min	-2592.195	3	1.678	15	0	1	0	4	-.022	4	-.004	3
309		3	max	2476.451	2	6.38	6	4.075	4	0	1	0	1	.006	2
310			min	-2592.322	3	1.497	15	0	1	0	4	-.02	4	-.005	3
311		4	max	2476.281	2	5.612	6	4.614	4	0	1	0	1	.003	2
312			min	-2592.45	3	1.317	15	0	1	0	4	-.019	4	-.006	3
313		5	max	2476.11	2	4.845	6	5.153	4	0	1	0	1	.001	2
314			min	-2592.578	3	1.137	15	0	1	0	4	-.017	4	-.007	3
315		6	max	2475.94	2	4.078	6	5.692	4	0	1	0	1	0	2
316			min	-2592.706	3	.956	15	0	1	0	4	-.014	4	-.008	3
317		7	max	2475.77	2	3.311	6	6.23	4	0	1	0	1	-.002	15
318			min	-2592.834	3	.747	12	0	1	0	4	-.012	4	-.008	3
319		8	max	2475.599	2	2.637	2	6.769	4	0	1	0	1	-.002	15
320			min	-2592.961	3	.448	12	0	1	0	4	-.009	4	-.009	3
321		9	max	2475.429	2	2.039	2	7.308	4	0	1	0	1	-.002	15
322			min	-2593.089	3	.149	12	0	1	0	4	-.006	4	-.009	3
323		10	max	2475.259	2	1.442	2	7.847	4	0	1	0	1	-.002	15
324			min	-2593.217	3	-.278	3	0	1	0	4	-.003	5	-.009	4
325		11	max	2475.088	2	.844	2	8.385	4	0	1	0	4	-.002	15
326			min	-2593.345	3	-.726	3	0	1	0	4	0	1	-.009	4
327		12	max	2474.918	2	.246	2	8.924	4	0	1	.004	4	-.002	15
328			min	-2593.472	3	-1.174	3	0	1	0	4	0	1	-.009	4
329		13	max	2474.748	2	-.306	15	9.463	4	0	1	.008	4	-.002	15
330			min	-2593.6	3	-1.623	3	0	1	0	4	0	1	-.009	4
331		14	max	2474.577	2	-.487	15	10.002	4	0	1	.012	4	-.002	15
332			min	-2593.728	3	-2.071	3	0	1	0	4	0	1	-.008	4
333		15	max	2474.407	2	-.667	15	10.54	4	0	1	.016	4	-.002	15
334			min	-2593.856	3	-2.827	4	0	1	0	4	0	1	-.007	4
335		16	max	2474.237	2	-.847	15	11.079	4	0	1	.021	4	-.001	15
336			min	-2593.983	3	-3.594	4	0	1	0	4	0	1	-.006	4
337		17	max	2474.066	2	-1.028	15	11.618	4	0	1	.026	4	-.001	15
338			min	-2594.111	3	-4.361	4	0	1	0	4	0	1	-.004	4
339		18	max	2473.896	2	-1.208	15	12.157	4	0	1	.031	4	0	15
340			min	-2594.239	3	-5.129	4	0	1	0	4	0	1	-.002	4
341		19	max	2473.726	2	-1.388	15	12.695	4	0	1	.036	4	0	1
342			min	-2594.367	3	-5.896	4	0	1	0	4	0	1	0	1
343	M8	1	max	3033.912	2	0	1	0	1	0	1	.026	4	0	1
344			min	-631.766	3	0	1	-244.688	4	0	1	0	1	0	1
345		2	max	3034.082	2	0	1	0	1	0	1	0	1	0	1
346			min	-631.638	3	0	1	-244.835	4	0	1	-.002	4	0	1
347		3	max	3034.253	2	0	1	0	1	0	1	0	1	0	1
348			min	-631.51	3	0	1	-244.983	4	0	1	-.03	4	0	1
349		4	max	3034.423	2	0	1	0	1	0	1	0	1	0	1
350			min	-631.382	3	0	1	-245.131	4	0	1	-.059	4	0	1
351		5	max	3034.594	2	0	1	0	1	0	1	0	1	0	1
352			min	-631.255	3	0	1	-245.278	4	0	1	-.087	4	0	1
353		6	max	3034.764	2	0	1	0	1	0	1	0	1	0	1
354			min	-631.127	3	0	1	-245.426	4	0	1	-.115	4	0	1
355		7	max	3034.934	2	0	1	0	1	0	1	0	1	0	1
356			min	-630.999	3	0	1	-245.574	4	0	1	-.143	4	0	1
357		8	max	3035.105	2	0	1	0	1	0	1	0	1	0	1
358			min	-630.871	3	0	1	-245.721	4	0	1	-.171	4	0	1
359		9	max	3035.275	2	0	1	0	1	0	1	0	1	0	1
360			min	-630.744	3	0	1	-245.869	4	0	1	-.199	4	0	1



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3035.445	2	0	1	0	1	0	1	0	1	0	1
362			min	-630.616	3	0	1	-246.016	4	0	1	-.228	4	0	1
363		11	max	3035.616	2	0	1	0	1	0	1	0	1	0	1
364			min	-630.488	3	0	1	-246.164	4	0	1	-.256	4	0	1
365		12	max	3035.786	2	0	1	0	1	0	1	0	1	0	1
366			min	-630.36	3	0	1	-246.312	4	0	1	-.284	4	0	1
367		13	max	3035.956	2	0	1	0	1	0	1	0	1	0	1
368			min	-630.233	3	0	1	-246.459	4	0	1	-.313	4	0	1
369		14	max	3036.127	2	0	1	0	1	0	1	0	1	0	1
370			min	-630.105	3	0	1	-246.607	4	0	1	-.341	4	0	1
371		15	max	3036.297	2	0	1	0	1	0	1	0	1	0	1
372			min	-629.977	3	0	1	-246.755	4	0	1	-.369	4	0	1
373		16	max	3036.467	2	0	1	0	1	0	1	0	1	0	1
374			min	-629.849	3	0	1	-246.902	4	0	1	-.398	4	0	1
375		17	max	3036.638	2	0	1	0	1	0	1	0	1	0	1
376			min	-629.721	3	0	1	-247.05	4	0	1	-.426	4	0	1
377		18	max	3036.808	2	0	1	0	1	0	1	0	1	0	1
378			min	-629.594	3	0	1	-247.198	4	0	1	-.454	4	0	1
379		19	max	3036.978	2	0	1	0	1	0	1	0	1	0	1
380			min	-629.466	3	0	1	-247.345	4	0	1	-.483	4	0	1
381	M10	1	max	1112.776	2	1.886	6	-.032	12	0	1	0	2	0	1
382			min	-1522.489	3	.427	15	-38.557	4	0	5	0	3	0	1
383		2	max	1113.204	2	1.829	6	-.032	12	0	1	0	10	0	15
384			min	-1522.167	3	.414	15	-38.931	4	0	5	-.011	4	0	6
385		3	max	1113.633	2	1.772	6	-.032	12	0	1	0	10	0	15
386			min	-1521.846	3	.401	15	-39.304	4	0	5	-.023	4	-.001	6
387		4	max	1114.061	2	1.715	6	-.032	12	0	1	0	12	0	15
388			min	-1521.524	3	.387	15	-39.677	4	0	5	-.034	4	-.002	6
389		5	max	1114.489	2	1.659	6	-.032	12	0	1	0	12	0	15
390			min	-1521.203	3	.374	15	-40.051	4	0	5	-.046	4	-.002	6
391		6	max	1114.918	2	1.602	6	-.032	12	0	1	0	12	0	15
392			min	-1520.882	3	.361	15	-40.424	4	0	5	-.057	4	-.003	6
393		7	max	1115.346	2	1.545	6	-.032	12	0	1	0	12	0	15
394			min	-1520.56	3	.347	15	-40.797	4	0	5	-.069	4	-.003	6
395		8	max	1115.775	2	1.488	6	-.032	12	0	1	0	12	0	15
396			min	-1520.239	3	.334	15	-41.171	4	0	5	-.081	4	-.003	6
397		9	max	1116.203	2	1.431	6	-.032	12	0	1	0	12	0	15
398			min	-1519.918	3	.321	15	-41.544	4	0	5	-.093	4	-.004	6
399		10	max	1116.632	2	1.383	2	-.032	12	0	1	0	12	0	15
400			min	-1519.596	3	.307	15	-41.917	4	0	5	-.105	4	-.004	6
401		11	max	1117.06	2	1.339	2	-.032	12	0	1	0	12	-.001	15
402			min	-1519.275	3	.288	12	-42.291	4	0	5	-.117	4	-.005	6
403		12	max	1117.489	2	1.295	2	-.032	12	0	1	0	12	-.001	15
404			min	-1518.954	3	.265	12	-42.664	4	0	5	-.13	4	-.005	6
405		13	max	1117.917	2	1.251	2	-.032	12	0	1	0	12	-.001	15
406			min	-1518.632	3	.243	12	-43.037	4	0	5	-.142	4	-.005	6
407		14	max	1118.346	2	1.206	2	-.032	12	0	1	0	12	-.001	15
408			min	-1518.311	3	.221	12	-43.411	4	0	5	-.155	4	-.006	6
409		15	max	1118.774	2	1.162	2	-.032	12	0	1	0	12	-.001	15
410			min	-1517.989	3	.199	12	-43.784	4	0	5	-.167	4	-.006	6
411		16	max	1119.203	2	1.118	2	-.032	12	0	1	0	12	-.001	15
412			min	-1517.668	3	.177	12	-44.157	4	0	5	-.18	4	-.006	6
413		17	max	1119.631	2	1.074	2	-.032	12	0	1	0	12	-.001	15
414			min	-1517.347	3	.155	12	-44.531	4	0	5	-.193	4	-.007	6
415		18	max	1120.06	2	1.029	2	-.032	12	0	1	0	12	-.002	15
416			min	-1517.025	3	.133	12	-44.904	4	0	5	-.206	4	-.007	2
417		19	max	1120.488	2	.985	2	-.032	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1516.704	3	.111	12	-45.277	4	0	5	-.219	4	-.007	2
419	M11	1	max	690.003	2	7.858	6	3.12	4	0	1	0	12	.007	2
420			min	-827.835	3	1.836	15	-.138	1	0	4	-.023	4	.002	15
421		2	max	689.833	2	7.09	6	3.659	4	0	1	0	12	.005	2
422			min	-827.963	3	1.656	15	-.138	1	0	4	-.022	4	0	12
423		3	max	689.662	2	6.323	6	4.198	4	0	1	0	12	.002	2
424			min	-828.091	3	1.475	15	-.138	1	0	4	-.02	4	0	3
425		4	max	689.492	2	5.556	6	4.737	4	0	1	0	12	0	2
426			min	-828.219	3	1.295	15	-.138	1	0	4	-.018	4	-.002	3
427		5	max	689.321	2	4.789	6	5.275	4	0	1	0	12	0	15
428			min	-828.347	3	1.115	15	-.138	1	0	4	-.016	4	-.003	3
429		6	max	689.151	2	4.021	6	5.814	4	0	1	0	12	-.001	15
430			min	-828.474	3	.934	15	-.138	1	0	4	-.014	4	-.005	4
431		7	max	688.981	2	3.254	6	6.353	4	0	1	0	12	-.002	15
432			min	-828.602	3	.754	15	-.138	1	0	4	-.011	4	-.007	4
433		8	max	688.81	2	2.487	6	6.892	4	0	1	0	12	-.002	15
434			min	-828.73	3	.574	15	-.138	1	0	4	-.009	4	-.008	4
435		9	max	688.64	2	1.72	6	7.43	4	0	1	0	12	-.002	15
436			min	-828.858	3	.393	15	-.138	1	0	4	-.006	4	-.009	4
437		10	max	688.47	2	.959	2	7.969	4	0	1	0	12	-.002	15
438			min	-828.985	3	.213	15	-.138	1	0	4	-.002	4	-.009	4
439		11	max	688.299	2	.361	2	8.508	4	0	1	.001	5	-.002	15
440			min	-829.113	3	-.143	3	-.138	1	0	4	0	1	-.01	4
441		12	max	688.129	2	-.148	15	9.047	4	0	1	.005	5	-.002	15
442			min	-829.241	3	-.592	3	-.138	1	0	4	0	1	-.01	4
443		13	max	687.959	2	-.328	15	9.585	4	0	1	.009	5	-.002	15
444			min	-829.369	3	-1.35	4	-.138	1	0	4	-.001	1	-.009	4
445		14	max	687.788	2	-.508	15	10.124	4	0	1	.013	5	-.002	15
446			min	-829.496	3	-2.117	4	-.138	1	0	4	-.001	1	-.008	4
447		15	max	687.618	2	-.689	15	10.663	4	0	1	.017	4	-.002	15
448			min	-829.624	3	-2.885	4	-.138	1	0	4	-.001	1	-.007	4
449		16	max	687.448	2	-.869	15	11.202	4	0	1	.022	4	-.001	15
450			min	-829.752	3	-3.652	4	-.138	1	0	4	-.001	1	-.006	4
451		17	max	687.277	2	-1.05	15	11.74	4	0	1	.026	4	-.001	15
452			min	-829.88	3	-4.419	4	-.138	1	0	4	-.001	1	-.004	4
453		18	max	687.107	2	-1.23	15	12.279	4	0	1	.031	4	0	15
454			min	-830.007	3	-5.186	4	-.138	1	0	4	-.001	1	-.002	4
455		19	max	686.937	2	-1.41	15	12.818	4	0	1	.037	4	0	1
456			min	-830.135	3	-5.953	4	-.138	1	0	4	-.001	1	0	1
457	M12	1	max	1102.578	1	0	1	8.842	1	0	1	.027	4	0	1
458			min	-170.707	3	0	1	-247.098	4	0	1	0	1	0	1
459		2	max	1102.748	1	0	1	8.842	1	0	1	0	1	0	1
460			min	-170.579	3	0	1	-247.246	4	0	1	-.002	4	0	1
461		3	max	1102.918	1	0	1	8.842	1	0	1	.001	1	0	1
462			min	-170.452	3	0	1	-247.394	4	0	1	-.03	4	0	1
463		4	max	1103.089	1	0	1	8.842	1	0	1	.002	1	0	1
464			min	-170.324	3	0	1	-247.541	4	0	1	-.059	4	0	1
465		5	max	1103.259	1	0	1	8.842	1	0	1	.003	1	0	1
466			min	-170.196	3	0	1	-247.689	4	0	1	-.087	4	0	1
467		6	max	1103.429	1	0	1	8.842	1	0	1	.004	1	0	1
468			min	-170.068	3	0	1	-247.837	4	0	1	-.116	4	0	1
469		7	max	1103.6	1	0	1	8.842	1	0	1	.005	1	0	1
470			min	-169.941	3	0	1	-247.984	4	0	1	-.144	4	0	1
471		8	max	1103.77	1	0	1	8.842	1	0	1	.006	1	0	1
472			min	-169.813	3	0	1	-248.132	4	0	1	-.173	4	0	1
473		9	max	1103.94	1	0	1	8.842	1	0	1	.007	1	0	1
474			min	-169.685	3	0	1	-248.279	4	0	1	-.201	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1104.111	1	0	1	8.842	1	0	1	.008	1	0	1
476			min	-169.557	3	0	1	-248.427	4	0	1	-.23	4	0	1
477		11	max	1104.281	1	0	1	8.842	1	0	1	.009	1	0	1
478			min	-169.43	3	0	1	-248.575	4	0	1	-.258	4	0	1
479		12	max	1104.451	1	0	1	8.842	1	0	1	.01	1	0	1
480			min	-169.302	3	0	1	-248.722	4	0	1	-.287	4	0	1
481		13	max	1104.622	1	0	1	8.842	1	0	1	.011	1	0	1
482			min	-169.174	3	0	1	-248.87	4	0	1	-.315	4	0	1
483		14	max	1104.792	1	0	1	8.842	1	0	1	.012	1	0	1
484			min	-169.046	3	0	1	-249.018	4	0	1	-.344	4	0	1
485		15	max	1104.962	1	0	1	8.842	1	0	1	.013	1	0	1
486			min	-168.919	3	0	1	-249.165	4	0	1	-.372	4	0	1
487		16	max	1105.133	1	0	1	8.842	1	0	1	.014	1	0	1
488			min	-168.791	3	0	1	-249.313	4	0	1	-.401	4	0	1
489		17	max	1105.303	1	0	1	8.842	1	0	1	.015	1	0	1
490			min	-168.663	3	0	1	-249.46	4	0	1	-.43	4	0	1
491		18	max	1105.473	1	0	1	8.842	1	0	1	.016	1	0	1
492			min	-168.535	3	0	1	-249.608	4	0	1	-.458	4	0	1
493		19	max	1105.644	1	0	1	8.842	1	0	1	.017	1	0	1
494			min	-168.407	3	0	1	-249.756	4	0	1	-.487	4	0	1
495	M1	1	max	157.857	1	835.562	3	44.037	5	0	2	.205	1	0	15
496			min	-10.857	5	-474.382	2	-85.802	1	0	3	-.085	5	-.015	2
497		2	max	158.462	1	834.588	3	45.279	5	0	2	.159	1	.235	2
498			min	-10.575	5	-475.68	2	-85.802	1	0	3	-.061	5	-.441	3
499		3	max	511.091	3	571.063	2	9.071	5	0	3	.114	1	.474	2
500			min	-298.321	2	-608.562	3	-85.414	1	0	2	-.037	5	-.864	3
501		4	max	511.545	3	569.765	2	10.312	5	0	3	.069	1	.176	1
502			min	-297.716	2	-609.536	3	-85.414	1	0	2	-.032	5	-.542	3
503		5	max	511.999	3	568.467	2	11.554	5	0	3	.024	1	-.003	15
504			min	-297.11	2	-610.509	3	-85.414	1	0	2	-.027	5	-.221	3
505		6	max	512.453	3	567.169	2	12.795	5	0	3	-.001	12	.102	3
506			min	-296.505	2	-611.483	3	-85.414	1	0	2	-.025	4	-.427	2
507		7	max	512.907	3	565.871	2	14.036	5	0	3	-.004	12	.425	3
508			min	-295.9	2	-612.457	3	-85.414	1	0	2	-.066	1	-.726	2
509		8	max	513.362	3	564.572	2	15.278	5	0	3	-.003	15	.748	3
510			min	-295.294	2	-613.43	3	-85.414	1	0	2	-.111	1	-1.024	2
511		9	max	525.63	3	51.192	2	52.083	5	0	9	.067	1	.874	3
512			min	-229.445	2	.392	15	-128.024	1	0	3	-.117	5	-1.172	2
513		10	max	526.084	3	49.894	2	53.324	5	0	9	0	10	.851	3
514			min	-228.839	2	0	5	-128.024	1	0	3	-.09	4	-1.199	2
515		11	max	526.538	3	48.596	2	54.566	5	0	9	-.004	12	.829	3
516			min	-228.234	2	-1.623	4	-128.024	1	0	3	-.076	4	-1.225	2
517		12	max	538.672	3	394.756	3	138.339	5	0	2	.11	1	.723	3
518			min	-162.334	2	-670.834	2	-83.576	1	0	3	-.192	5	-1.086	2
519		13	max	539.126	3	393.782	3	139.58	5	0	2	.066	1	.515	3
520			min	-161.728	2	-672.132	2	-83.576	1	0	3	-.119	5	-.731	2
521		14	max	539.58	3	392.809	3	140.822	5	0	2	.022	1	.307	3
522			min	-161.123	2	-673.43	2	-83.576	1	0	3	-.045	5	-.376	2
523		15	max	540.035	3	391.835	3	142.063	5	0	2	.03	5	.1	3
524			min	-160.517	2	-674.728	2	-83.576	1	0	3	-.022	1	-.041	1
525		16	max	540.489	3	390.861	3	143.305	5	0	2	.105	5	.336	2
526			min	-159.912	2	-676.026	2	-83.576	1	0	3	-.066	1	-.106	3
527		17	max	540.943	3	389.888	3	144.546	5	0	2	.181	5	.693	2
528			min	-159.307	2	-677.325	2	-83.576	1	0	3	-.111	1	-.312	3
529		18	max	20.325	5	682.662	2	-5.374	12	0	5	.176	5	.349	2
530			min	-158.746	1	-316.431	3	-109.147	4	0	2	-.158	1	-.154	3
531		19	max	20.607	5	681.364	2	-5.374	12	0	5	.13	5	.013	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-158.141	1	-317.405	3	-107.906	4	0	2	-.206	1	-.011	2
533	M5	max	345.225	1	2780.999	3	85.731	5	0	1	0	1	.031	2
534		min	11.996	12	-1619.685	2	0	1	0	4	-.184	4	0	15
535		max	345.83	1	2780.025	3	86.973	5	0	1	0	1	.886	2
536		min	12.299	12	-1620.984	2	0	1	0	4	-.138	4	-1.466	3
537		max	1631.143	3	1692.472	2	53.268	4	0	4	0	1	1.702	2
538		min	-1015.041	2	-1919.06	3	0	1	0	1	-.093	4	-2.877	3
539		max	1631.597	3	1691.174	2	54.51	4	0	4	0	1	.809	2
540		min	-1014.436	2	-1920.033	3	0	1	0	1	-.064	4	-1.864	3
541		max	1632.052	3	1689.876	2	55.751	4	0	4	0	1	.013	9
542		min	-1013.83	2	-1921.007	3	0	1	0	1	-.035	4	-.85	3
543		max	1632.506	3	1688.577	2	56.993	4	0	4	0	1	.164	3
544		min	-1013.225	2	-1921.981	3	0	1	0	1	-.006	5	-.974	2
545		max	1632.96	3	1687.279	2	58.234	4	0	4	.025	4	1.178	3
546		min	-1012.62	2	-1922.954	3	0	1	0	1	0	1	-1.865	2
547		max	1633.414	3	1685.981	2	59.476	4	0	4	.056	4	2.193	3
548		min	-1012.014	2	-1923.928	3	0	1	0	1	0	1	-2.755	2
549		max	1651.004	3	171.748	2	169.438	4	0	1	0	1	2.526	3
550		min	-873.409	2	.391	15	0	1	0	1	-.171	4	-3.139	2
551		max	1651.458	3	170.45	2	170.68	4	0	1	0	1	2.442	3
552		min	-872.804	2	0	15	0	1	0	1	-.081	4	-3.229	2
553		max	1651.912	3	169.151	2	171.921	4	0	1	.01	4	2.359	3
554		min	-872.199	2	-1.498	6	0	1	0	1	0	1	-3.319	2
555		max	1669.769	3	1223.594	3	198.118	4	0	1	0	1	2.07	3
556		min	-733.696	2	-2033.574	2	0	1	0	4	-.278	4	-2.971	2
557		max	1670.223	3	1222.62	3	199.36	4	0	1	0	1	1.424	3
558		min	-733.091	2	-2034.872	2	0	1	0	4	-.173	4	-1.898	2
559		max	1670.677	3	1221.647	3	200.601	4	0	1	0	1	.779	3
560		min	-732.486	2	-2036.17	2	0	1	0	4	-.067	4	-.824	2
561		max	1671.131	3	1220.673	3	201.843	4	0	1	.039	4	.251	2
562		min	-731.88	2	-2037.469	2	0	1	0	4	0	1	-.003	13
563		max	1671.585	3	1219.699	3	203.084	4	0	1	.146	4	1.327	2
564		min	-731.275	2	-2038.767	2	0	1	0	4	0	1	-.509	3
565		max	1672.039	3	1218.726	3	204.325	4	0	1	.253	4	2.403	2
566		min	-730.67	2	-2040.065	2	0	1	0	4	0	1	-1.152	3
567		max	-12.865	12	2305.286	2	0	1	0	4	.281	4	1.238	2
568		min	-345.263	1	-1093.193	3	-24.408	5	0	1	0	1	-.603	3
569		max	-12.562	12	2303.987	2	0	1	0	4	.269	4	.022	2
570		min	-344.657	1	-1094.166	3	-23.167	5	0	1	0	1	-.026	3
571	M9	max	157.857	1	835.562	3	85.802	1	0	3	-.013	12	0	15
572		min	7.59	12	-474.382	2	5.475	12	0	4	-.205	1	-.015	2
573		max	158.462	1	834.588	3	85.802	1	0	3	-.01	12	.235	2
574		min	7.893	12	-475.68	2	5.475	12	0	4	-.159	1	-.441	3
575		max	511.091	3	571.063	2	85.414	1	0	2	-.007	12	.474	2
576		min	-298.321	2	-608.562	3	5.441	12	0	3	-.114	1	-.864	3
577		max	511.545	3	569.765	2	85.414	1	0	2	-.004	12	.176	1
578		min	-297.716	2	-609.536	3	5.441	12	0	3	-.069	1	-.542	3
579		max	511.999	3	568.467	2	85.414	1	0	2	-.002	12	-.003	15
580		min	-297.11	2	-610.509	3	5.441	12	0	3	-.035	4	-.221	3
581		max	512.453	3	567.169	2	85.414	1	0	2	.021	1	.102	3
582		min	-296.505	2	-611.483	3	5.441	12	0	3	-.017	5	-.427	2
583		max	512.907	3	565.871	2	85.414	1	0	2	.066	1	.425	3
584		min	-295.9	2	-612.457	3	5.441	12	0	3	-.005	5	-.726	2
585		max	513.362	3	564.572	2	85.414	1	0	2	.111	1	.748	3
586		min	-295.294	2	-613.43	3	5.441	12	0	3	.006	15	-1.024	2
587		max	525.63	3	51.192	2	128.024	1	0	3	-.004	12	.874	3
588		min	-229.445	2	.399	15	7.807	12	0	9	-.141	4	-1.172	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	526.084	3	49.894	2	128.024	1	0	3	0	1	.851	3
590		min	-228.839	2	.007	15	7.807	12	0	9	-.089	4	-1.199	2
591	11	max	526.538	3	48.596	2	128.024	1	0	3	.068	1	.829	3
592		min	-228.234	2	-1.573	6	7.807	12	0	9	-.052	5	-1.225	2
593	12	max	538.672	3	394.756	3	169.371	4	0	3	-.006	12	.723	3
594		min	-162.334	2	-670.834	2	4.874	12	0	2	-.233	4	-1.086	2
595	13	max	539.126	3	393.782	3	170.613	4	0	3	-.004	12	.515	3
596		min	-161.728	2	-672.132	2	4.874	12	0	2	-.143	4	-.731	2
597	14	max	539.58	3	392.809	3	171.854	4	0	3	-.001	12	.307	3
598		min	-161.123	2	-673.43	2	4.874	12	0	2	-.053	4	-.376	2
599	15	max	540.035	3	391.835	3	173.096	4	0	3	.038	4	.1	3
600		min	-160.517	2	-674.728	2	4.874	12	0	2	.001	12	-.041	1
601	16	max	540.489	3	390.861	3	174.337	4	0	3	.13	4	.336	2
602		min	-159.912	2	-676.026	2	4.874	12	0	2	.004	12	-.106	3
603	17	max	540.943	3	389.888	3	175.579	4	0	3	.222	4	.693	2
604		min	-159.307	2	-677.325	2	4.874	12	0	2	.006	12	-.312	3
605	18	max	-7.61	12	682.662	2	92.042	1	0	2	.234	4	.349	2
606		min	-158.746	1	-316.431	3	-76.312	5	0	3	.009	12	-.154	3
607	19	max	-7.307	12	681.364	2	92.042	1	0	2	.206	1	.013	3
608		min	-158.141	1	-317.405	3	-75.071	5	0	3	.012	12	-.011	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.123	2	.009	3	1.017e-2	2	NC	1	NC	1
2			min	-.552	4	-.022	3	-.005	2	-1.964e-3	3	NC	1	NC	1
3		2	max	0	1	.268	3	.027	1	1.153e-2	2	NC	5	NC	2
4			min	-.552	4	-.03	1	-.015	5	-1.933e-3	3	745.562	3	8114.981	1
5		3	max	0	1	.503	3	.064	1	1.289e-2	2	NC	5	NC	3
6			min	-.552	4	-.136	1	-.018	5	-1.901e-3	3	411.765	3	3368.818	1
7		4	max	0	1	.646	3	.096	1	1.425e-2	2	NC	5	NC	3
8			min	-.553	4	-.194	1	-.013	5	-1.87e-3	3	323.496	3	2247.356	1
9		5	max	0	1	.68	3	.112	1	1.561e-2	2	NC	5	NC	3
10			min	-.553	4	-.194	1	-.003	5	-1.838e-3	3	307.746	3	1927.338	1
11		6	max	0	1	.608	3	.108	1	1.697e-2	2	NC	5	NC	3
12			min	-.553	4	-.138	1	.004	10	-1.807e-3	3	343.142	3	2012.745	1
13		7	max	0	1	.451	3	.083	1	1.832e-2	2	NC	5	NC	3
14			min	-.553	4	-.04	1	0	10	-1.775e-3	3	457.118	3	2601.668	1
15		8	max	0	1	.251	3	.047	1	1.968e-2	2	NC	2	NC	2
16			min	-.553	4	.002	15	-.005	10	-1.744e-3	3	792.171	3	4672.659	1
17		9	max	0	1	.221	2	.028	3	2.104e-2	2	NC	4	NC	1
18			min	-.553	4	.004	15	-.012	2	-1.713e-3	3	2212.193	2	NC	1
19		10	max	0	1	.271	2	.028	3	2.24e-2	2	NC	3	NC	1
20			min	-.553	4	-.013	3	-.019	2	-1.681e-3	3	1460.41	2	NC	1
21		11	max	0	12	.221	2	.028	3	2.104e-2	2	NC	4	NC	1
22			min	-.553	4	.004	15	-.012	5	-1.713e-3	3	2212.193	2	NC	1
23		12	max	0	12	.251	3	.047	1	1.968e-2	2	NC	2	NC	2
24			min	-.553	4	.002	15	-.012	5	-1.744e-3	3	792.171	3	4672.659	1
25		13	max	0	12	.451	3	.083	1	1.832e-2	2	NC	5	NC	3
26			min	-.553	4	-.04	1	-.004	5	-1.775e-3	3	457.118	3	2601.668	1
27		14	max	0	12	.608	3	.108	1	1.697e-2	2	NC	5	NC	3
28			min	-.553	4	-.138	1	.004	10	-1.807e-3	3	343.142	3	2012.745	1
29		15	max	0	12	.68	3	.112	1	1.561e-2	2	NC	5	NC	3
30			min	-.553	4	-.194	1	.005	10	-1.838e-3	3	307.746	3	1927.338	1
31		16	max	0	12	.646	3	.096	1	1.425e-2	2	NC	5	NC	3
32			min	-.553	4	-.194	1	.005	10	-1.87e-3	3	323.496	3	2247.356	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.503	3	.064	1	1.289e-2	2	NC	5	NC	3
34		min	-5.553	4	-.136	1	.002	10	-1.901e-3	3	411.765	3	3368.818	1
35	18	max	0	12	.268	3	.027	1	1.153e-2	2	NC	5	NC	2
36		min	-5.553	4	-.03	1	-.001	10	-1.933e-3	3	745.562	3	8114.981	1
37	19	max	0	12	.123	2	.009	3	1.017e-2	2	NC	1	NC	1
38		min	-5.553	4	-.022	3	-.005	2	-1.964e-3	3	NC	1	NC	1
39	M14	1	max	0	.262	3	.008	3	5.933e-3	2	NC	1	NC	1
40		min	-.421	4	-.392	2	-.005	2	-4.65e-3	3	NC	1	NC	1
41	2	max	0	1	.557	3	.018	1	7.048e-3	2	NC	5	NC	1
42		min	-.421	4	-.664	2	-.022	5	-5.614e-3	3	730.251	3	9130.581	5
43	3	max	0	1	.81	3	.051	1	8.163e-3	2	NC	5	NC	2
44		min	-.421	4	-.902	2	-.027	5	-6.578e-3	3	393.99	3	4273.333	1
45	4	max	0	1	.989	3	.082	1	9.277e-3	2	NC	15	NC	3
46		min	-.421	4	-1.081	2	-.019	5	-7.542e-3	3	296.869	3	2658.268	1
47	5	max	0	1	1.08	3	.099	1	1.039e-2	2	NC	15	NC	3
48		min	-.421	4	-1.189	2	-.003	5	-8.505e-3	3	263.758	3	2193.741	1
49	6	max	0	1	1.084	3	.097	1	1.151e-2	2	NC	15	NC	3
50		min	-.421	4	-1.226	2	.003	10	-9.469e-3	3	259.092	2	2235.186	1
51	7	max	0	1	1.013	3	.077	1	1.262e-2	2	NC	15	NC	3
52		min	-.421	4	-1.201	2	0	10	-1.043e-2	3	267.156	2	2838.153	1
53	8	max	0	1	.899	3	.044	4	1.374e-2	2	NC	15	NC	2
54		min	-.421	4	-1.135	2	-.005	10	-1.14e-2	3	290.694	2	4847.383	4
55	9	max	0	1	.786	3	.029	4	1.485e-2	2	NC	5	NC	1
56		min	-.421	4	-1.063	2	-.011	2	-1.236e-2	3	322.335	2	7299.377	4
57	10	max	0	1	.732	3	.025	3	1.597e-2	2	NC	5	NC	1
58		min	-.421	4	-1.026	2	-.018	2	-1.332e-2	3	340.762	2	NC	1
59	11	max	0	12	.786	3	.025	3	1.485e-2	2	NC	5	NC	1
60		min	-.421	4	-1.063	2	-.022	5	-1.236e-2	3	322.335	2	9737.827	5
61	12	max	0	12	.899	3	.044	1	1.374e-2	2	NC	15	NC	2
62		min	-.422	4	-1.135	2	-.026	5	-1.14e-2	3	290.694	2	5017.146	1
63	13	max	0	12	1.013	3	.077	1	1.262e-2	2	NC	15	NC	3
64		min	-.422	4	-1.201	2	-.016	5	-1.043e-2	3	267.156	2	2838.153	1
65	14	max	0	12	1.084	3	.097	1	1.151e-2	2	NC	15	NC	3
66		min	-.422	4	-1.226	2	0	15	-9.469e-3	3	259.092	2	2235.186	1
67	15	max	0	12	1.08	3	.099	1	1.039e-2	2	NC	15	NC	3
68		min	-.422	4	-1.189	2	.005	10	-8.505e-3	3	263.758	3	2193.741	1
69	16	max	0	12	.989	3	.082	1	9.277e-3	2	NC	15	NC	3
70		min	-.422	4	-1.081	2	.004	10	-7.542e-3	3	296.869	3	2658.268	1
71	17	max	0	12	.81	3	.051	1	8.163e-3	2	NC	5	NC	2
72		min	-.422	4	-.902	2	.001	10	-6.578e-3	3	393.99	3	4273.333	1
73	18	max	0	12	.557	3	.03	4	7.048e-3	2	NC	5	NC	1
74		min	-.422	4	-.664	2	-.002	10	-5.614e-3	3	730.251	3	7094.264	4
75	19	max	0	12	.262	3	.008	3	5.933e-3	2	NC	1	NC	1
76		min	-.422	4	-.392	2	-.005	2	-4.65e-3	3	NC	1	NC	1
77	M15	1	max	0	.267	3	.008	3	3.949e-3	3	NC	1	NC	1
78		min	-.348	4	-.392	2	-.004	2	-6.171e-3	2	NC	1	NC	1
79	2	max	0	12	.459	3	.018	1	4.77e-3	3	NC	5	NC	1
80		min	-.348	4	-.734	2	-.03	5	-7.335e-3	2	631.262	2	6806.744	5
81	3	max	0	12	.627	3	.051	1	5.591e-3	3	NC	5	NC	2
82		min	-.348	4	-1.028	2	-.037	5	-8.5e-3	2	339.281	2	4258.661	1
83	4	max	0	12	.756	3	.082	1	6.413e-3	3	NC	15	NC	3
84		min	-.348	4	-1.242	2	-.027	5	-9.664e-3	2	253.966	2	2650.453	1
85	5	max	0	12	.836	3	.099	1	7.234e-3	3	NC	15	NC	3
86		min	-.348	4	-1.359	2	-.007	5	-1.083e-2	2	223.368	2	2187.253	1
87	6	max	0	12	.868	3	.097	1	8.056e-3	3	NC	15	NC	3
88		min	-.348	4	-1.377	2	.004	10	-1.199e-2	2	219.164	2	2227.52	1
89	7	max	0	12	.857	3	.077	1	8.877e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.348	4	-1.313	2	0	10	-1.316e-2	2	234.352	2	2824.708	1
91	8	max	0	12	.817	3	.053	4	9.698e-3	3	NC	15	NC	2
92		min	-.348	4	-1.199	2	-.004	10	-1.432e-2	2	267.63	2	4061.457	4
93	9	max	0	12	.771	3	.036	4	1.052e-2	3	NC	5	NC	1
94		min	-.348	4	-1.081	2	-.01	2	-1.549e-2	2	313.231	2	5888.444	4
95	10	max	0	1	.747	3	.023	3	1.134e-2	3	NC	5	NC	1
96		min	-.348	4	-1.025	2	-.017	2	-1.665e-2	2	341.135	2	NC	1
97	11	max	0	1	.771	3	.024	3	1.052e-2	3	NC	5	NC	1
98		min	-.348	4	-1.081	2	-.029	5	-1.549e-2	2	313.231	2	7369.327	5
99	12	max	0	1	.817	3	.044	1	9.698e-3	3	NC	15	NC	2
100		min	-.348	4	-1.199	2	-.034	5	-1.432e-2	2	267.63	2	4973.008	1
101	13	max	0	1	.857	3	.077	1	8.877e-3	3	NC	15	NC	3
102		min	-.348	4	-1.313	2	-.023	5	-1.316e-2	2	234.352	2	2824.708	1
103	14	max	0	1	.868	3	.097	1	8.056e-3	3	NC	15	NC	3
104		min	-.348	4	-1.377	2	-.002	5	-1.199e-2	2	219.164	2	2227.52	1
105	15	max	0	1	.836	3	.099	1	7.234e-3	3	NC	15	NC	3
106		min	-.348	4	-1.359	2	.005	10	-1.083e-2	2	223.368	2	2187.253	1
107	16	max	0	1	.756	3	.082	1	6.413e-3	3	NC	15	NC	3
108		min	-.348	4	-1.242	2	.004	10	-9.664e-3	2	253.966	2	2650.453	1
109	17	max	0	1	.627	3	.057	4	5.591e-3	3	NC	5	NC	2
110		min	-.348	4	-1.028	2	.002	10	-8.5e-3	2	339.281	2	3775.724	4
111	18	max	0	1	.459	3	.038	4	4.77e-3	3	NC	5	NC	1
112		min	-.348	4	-.734	2	-.001	10	-7.335e-3	2	631.262	2	5596.369	4
113	19	max	0	1	.267	3	.008	3	3.949e-3	3	NC	1	NC	1
114		min	-.348	4	-.392	2	-.004	2	-6.171e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.11	.007	3	7.158e-3	3	NC	1	NC	1
116		min	-.135	4	-.089	3	-.004	2	-8.544e-3	2	NC	1	NC	1
117	2	max	0	12	.002	12	.027	1	8.31e-3	3	NC	5	NC	2
118		min	-.135	4	-.107	2	-.023	5	-9.522e-3	2	994.53	2	8156.834	1
119	3	max	0	12	.07	3	.064	1	9.461e-3	3	NC	5	NC	3
120		min	-.135	4	-.28	2	-.029	5	-1.05e-2	2	554.173	2	3373.085	1
121	4	max	0	12	.104	3	.097	1	1.061e-2	3	NC	5	NC	3
122		min	-.135	4	-.378	2	-.023	5	-1.148e-2	2	442.676	2	2244.566	1
123	5	max	0	12	.098	3	.113	1	1.176e-2	3	NC	5	NC	3
124		min	-.135	4	-.388	2	-.009	5	-1.246e-2	2	433.695	2	1920.174	1
125	6	max	0	12	.053	3	.108	1	1.292e-2	3	NC	5	NC	3
126		min	-.135	4	-.313	2	.005	15	-1.344e-2	2	511.155	2	1998.443	1
127	7	max	0	12	0	15	.084	1	1.407e-2	3	NC	5	NC	3
128		min	-.135	4	-.17	2	.002	10	-1.442e-2	2	770.987	2	2566.977	1
129	8	max	0	12	.033	1	.048	1	1.522e-2	3	NC	3	NC	2
130		min	-.135	4	-.11	3	-.003	10	-1.539e-2	2	2045.544	2	4532.14	1
131	9	max	0	12	.16	2	.026	4	1.637e-2	3	NC	4	NC	1
132		min	-.135	4	-.188	3	-.008	2	-1.637e-2	2	2192.594	3	8371.538	4
133	10	max	0	1	.23	2	.02	3	1.752e-2	3	NC	4	NC	1
134		min	-.135	4	-.222	3	-.015	2	-1.735e-2	2	1625.564	3	NC	1
135	11	max	0	1	.16	2	.021	3	1.637e-2	3	NC	4	NC	1
136		min	-.135	4	-.188	3	-.018	5	-1.637e-2	2	2192.594	3	NC	1
137	12	max	0	1	.033	1	.048	1	1.522e-2	3	NC	3	NC	2
138		min	-.135	4	-.11	3	-.019	5	-1.539e-2	2	2045.544	2	4532.14	1
139	13	max	0	1	0	15	.084	1	1.407e-2	3	NC	5	NC	3
140		min	-.135	4	-.17	2	-.009	5	-1.442e-2	2	770.987	2	2566.977	1
141	14	max	0	1	.053	3	.108	1	1.292e-2	3	NC	5	NC	3
142		min	-.135	4	-.313	2	.005	15	-1.344e-2	2	511.155	2	1998.443	1
143	15	max	0	1	.098	3	.113	1	1.176e-2	3	NC	5	NC	3
144		min	-.134	4	-.388	2	.007	10	-1.246e-2	2	433.695	2	1920.174	1
145	16	max	0	1	.104	3	.097	1	1.061e-2	3	NC	5	NC	3
146		min	-.134	4	-.378	2	.006	10	-1.148e-2	2	442.676	2	2244.566	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.07	3	.064	1	9.461e-3	3	NC	5	NC	3
148			min	-.134	4	-.28	2	.003	10	-1.05e-2	2	554.173	2	3373.085	1
149		18	max	0	1	.002	12	.034	4	8.31e-3	3	NC	5	NC	2
150			min	-.134	4	-.107	2	0	10	-9.522e-3	2	994.53	2	6296.428	4
151		19	max	0	1	.11	2	.007	3	7.158e-3	3	NC	1	NC	1
152			min	-.134	4	-.089	3	-.004	2	-8.544e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.008	2	.007	1	1.398e-3	5	NC	1	NC	2
154			min	-.009	3	-.013	3	-.52	4	-1.79e-4	1	8140.005	2	120.583	4
155		2	max	.006	2	.007	2	.006	1	1.474e-3	5	NC	1	NC	1
156			min	-.009	3	-.012	3	-.478	4	-1.68e-4	1	9299.251	2	131.266	4
157		3	max	.006	2	.006	2	.006	1	1.551e-3	5	NC	1	NC	1
158			min	-.008	3	-.012	3	-.436	4	-1.57e-4	1	NC	1	143.947	4
159		4	max	.006	2	.005	2	.005	1	1.627e-3	5	NC	1	NC	1
160			min	-.008	3	-.011	3	-.394	4	-1.46e-4	1	NC	1	159.145	4
161		5	max	.005	2	.004	2	.004	1	1.703e-3	5	NC	1	NC	1
162			min	-.007	3	-.011	3	-.353	4	-1.35e-4	1	NC	1	177.564	4
163		6	max	.005	2	.003	2	.004	1	1.779e-3	5	NC	1	NC	1
164			min	-.007	3	-.01	3	-.313	4	-1.24e-4	1	NC	1	200.18	4
165		7	max	.004	2	.002	2	.003	1	1.855e-3	5	NC	1	NC	1
166			min	-.006	3	-.009	3	-.275	4	-1.13e-4	1	NC	1	228.375	4
167		8	max	.004	2	.002	2	.003	1	1.931e-3	5	NC	1	NC	1
168			min	-.006	3	-.009	3	-.237	4	-1.02e-4	1	NC	1	264.162	4
169		9	max	.004	2	0	2	.003	1	2.008e-3	5	NC	1	NC	1
170			min	-.005	3	-.008	3	-.202	4	-9.102e-5	1	NC	1	310.563	4
171		10	max	.003	2	0	2	.002	1	2.088e-3	4	NC	1	NC	1
172			min	-.005	3	-.008	3	-.168	4	-8.003e-5	1	NC	1	372.296	4
173		11	max	.003	2	0	2	.002	1	2.167e-3	4	NC	1	NC	1
174			min	-.004	3	-.007	3	-.137	4	-6.903e-5	1	NC	1	457.054	4
175		12	max	.003	2	0	2	.001	1	2.247e-3	4	NC	1	NC	1
176			min	-.004	3	-.006	3	-.108	4	-5.804e-5	1	NC	1	578.104	4
177		13	max	.002	2	0	15	0	1	2.327e-3	4	NC	1	NC	1
178			min	-.003	3	-.005	3	-.083	4	-4.704e-5	1	NC	1	759.968	4
179		14	max	.002	2	0	15	0	1	2.407e-3	4	NC	1	NC	1
180			min	-.003	3	-.005	3	-.06	4	-3.605e-5	1	NC	1	1052.279	4
181		15	max	.001	2	0	15	0	1	2.487e-3	4	NC	1	NC	1
182			min	-.002	3	-.004	3	-.04	4	-2.505e-5	1	NC	1	1568.653	4
183		16	max	.001	2	0	15	0	1	2.567e-3	4	NC	1	NC	1
184			min	-.002	3	-.003	3	-.024	4	-1.405e-5	1	NC	1	2620.042	4
185		17	max	0	2	0	15	0	1	2.647e-3	4	NC	1	NC	1
186			min	-.001	3	-.002	3	-.012	4	-3.058e-6	1	NC	1	5341.281	4
187		18	max	0	2	0	15	0	1	2.727e-3	4	NC	1	NC	1
188			min	0	3	-.001	3	-.004	4	1.086e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.807e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	8.347e-7	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-3.028e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.856e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	1.281e-5	1	NC	1	NC	1
194			min	0	2	-.002	6	0	12	-8.785e-5	5	NC	1	NC	1
195		3	max	0	3	0	15	.026	4	5.153e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	1.84e-6	12	NC	1	6864.576	14
197		4	max	.001	3	-.001	15	.037	4	1.116e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	2.911e-6	12	NC	1	4744.075	14
199		5	max	.002	3	-.002	15	.048	4	1.716e-3	4	NC	1	NC	1
200			min	-.001	2	-.007	6	0	12	3.983e-6	12	NC	1	3684.588	14
201		6	max	.002	3	-.002	15	.058	4	2.317e-3	4	NC	1	NC	1
202			min	-.002	2	-.009	6	0	12	5.054e-6	12	NC	1	3048.17	14
203		7	max	.002	3	-.002	15	.067	4	2.917e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.01	6	0	12	6.125e-6	12	8753.833	6	2621.944	14
205		8	max	.003	3	-.003	15	.076	4	3.518e-3	4	NC	1	NC	1
206			min	-.002	2	-.012	6	0	12	7.197e-6	12	7849.535	6	2314.487	14
207		9	max	.003	3	-.003	15	.084	4	4.118e-3	4	NC	2	NC	1
208			min	-.003	2	-.012	6	0	12	8.268e-6	12	7313.745	6	2079.93	14
209		10	max	.004	3	-.003	15	.092	4	4.718e-3	4	NC	2	NC	1
210			min	-.003	2	-.013	6	0	12	9.339e-6	12	7052.841	6	1892.684	14
211		11	max	.004	3	-.003	15	.1	4	5.319e-3	4	NC	2	NC	1
212			min	-.003	2	-.013	6	0	12	1.041e-5	12	7028.288	6	1737.336	14
213		12	max	.004	3	-.003	15	.108	4	5.919e-3	4	NC	2	NC	1
214			min	-.004	2	-.013	6	0	12	1.148e-5	12	7241.853	6	1604.079	14
215		13	max	.005	3	-.003	15	.117	4	6.52e-3	4	NC	1	NC	1
216			min	-.004	2	-.012	6	0	12	1.255e-5	12	7738.166	6	1486.433	14
217		14	max	.005	3	-.002	15	.126	4	7.12e-3	4	NC	1	NC	1
218			min	-.004	2	-.011	6	0	12	1.362e-5	12	8627.96	6	1380.028	14
219		15	max	.006	3	-.002	15	.135	4	7.721e-3	4	NC	1	NC	1
220			min	-.005	2	-.009	6	0	12	1.47e-5	12	NC	1	1281.904	14
221		16	max	.006	3	-.001	15	.145	4	8.321e-3	4	NC	1	NC	1
222			min	-.005	2	-.007	6	0	12	1.577e-5	12	NC	1	1190.092	14
223		17	max	.006	3	0	15	.156	4	8.922e-3	4	NC	1	NC	1
224			min	-.005	2	-.006	1	0	12	1.684e-5	12	NC	1	1103.334	14
225		18	max	.007	3	0	15	.169	4	9.522e-3	4	NC	1	NC	1
226			min	-.006	2	-.004	1	0	12	1.791e-5	12	NC	1	1020.887	14
227		19	max	.007	3	0	5	.183	4	1.012e-2	4	NC	1	NC	1
228			min	-.006	2	-.002	1	0	12	1.898e-5	12	NC	1	942.373	14
229	M4	1	max	.003	1	.006	2	0	12	5.214e-5	1	NC	1	NC	3
230			min	0	3	-.007	3	-.183	4	3.315e-6	12	NC	1	135.803	4
231		2	max	.002	1	.005	2	0	12	5.214e-5	1	NC	1	NC	2
232			min	0	3	-.007	3	-.168	4	3.315e-6	12	NC	1	147.631	4
233		3	max	.002	1	.005	2	0	12	5.214e-5	1	NC	1	NC	2
234			min	0	3	-.007	3	-.153	4	3.315e-6	12	NC	1	161.711	4
235		4	max	.002	1	.005	2	0	12	5.214e-5	1	NC	1	NC	2
236			min	0	3	-.006	3	-.139	4	3.315e-6	12	NC	1	178.625	4
237		5	max	.002	1	.004	2	0	12	5.214e-5	1	NC	1	NC	2
238			min	0	3	-.006	3	-.125	4	3.315e-6	12	NC	1	199.168	4
239		6	max	.002	1	.004	2	0	12	5.214e-5	1	NC	1	NC	2
240			min	0	3	-.005	3	-.111	4	3.315e-6	12	NC	1	224.441	4
241		7	max	.002	1	.004	2	0	12	5.214e-5	1	NC	1	NC	2
242			min	0	3	-.005	3	-.097	4	3.315e-6	12	NC	1	256.007	4
243		8	max	.002	1	.003	2	0	12	5.214e-5	1	NC	1	NC	2
244			min	0	3	-.005	3	-.084	4	3.315e-6	12	NC	1	296.144	4
245		9	max	.001	1	.003	2	0	12	5.214e-5	1	NC	1	NC	2
246			min	0	3	-.004	3	-.071	4	3.315e-6	12	NC	1	348.285	4
247		10	max	.001	1	.003	2	0	12	5.214e-5	1	NC	1	NC	1
248			min	0	3	-.004	3	-.059	4	3.315e-6	12	NC	1	417.792	4
249		11	max	.001	1	.002	2	0	12	5.214e-5	1	NC	1	NC	1
250			min	0	3	-.003	3	-.048	4	3.315e-6	12	NC	1	513.437	4
251		12	max	.001	1	.002	2	0	12	5.214e-5	1	NC	1	NC	1
252			min	0	3	-.003	3	-.038	4	3.315e-6	12	NC	1	650.397	4
253		13	max	0	1	.002	2	0	12	5.214e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.029	4	3.315e-6	12	NC	1	856.832	4
255		14	max	0	1	.002	2	0	12	5.214e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	-.021	4	3.315e-6	12	NC	1	1190.028	4
257		15	max	0	1	.001	2	0	12	5.214e-5	1	NC	1	NC	1
258			min	0	3	-.002	3	-.014	4	3.315e-6	12	NC	1	1782.017	4
259		16	max	0	1	0	2	0	12	5.214e-5	1	NC	1	NC	1
260			min	0	3	-.001	3	-.008	4	3.315e-6	12	NC	1	2997.717	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	12	5.214e-5	1	NC	1	NC	1
262		min	0	3	0	3	-.004	4	3.315e-6	12	NC	1	6189.75	4
263	18	max	0	1	0	2	0	12	5.214e-5	1	NC	1	NC	1
264		min	0	3	0	3	-.001	4	3.315e-6	12	NC	1	NC	1
265	19	max	0	1	0	1	0	1	5.214e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	3.315e-6	12	NC	1	NC	1
267	M6	1	max	.021	2	.028	2	0	1.469e-3	4	NC	4	NC	1
268		min	-.03	3	-.04	3	-.525	4	0	1	1570.999	3	119.505	4
269	2	max	.02	2	.026	2	0	1	1.543e-3	4	NC	4	NC	1
270		min	-.028	3	-.038	3	-.482	4	0	1	1667.157	3	130.095	4
271	3	max	.019	2	.023	2	0	1	1.617e-3	4	NC	4	NC	1
272		min	-.026	3	-.035	3	-.44	4	0	1	1775.815	3	142.665	4
273	4	max	.018	2	.021	2	0	1	1.692e-3	4	NC	4	NC	1
274		min	-.025	3	-.033	3	-.398	4	0	1	1899.527	3	157.73	4
275	5	max	.017	2	.019	2	0	1	1.766e-3	4	NC	4	NC	1
276		min	-.023	3	-.031	3	-.356	4	0	1	2041.575	3	175.989	4
277	6	max	.015	2	.017	2	0	1	1.841e-3	4	NC	4	NC	1
278		min	-.022	3	-.028	3	-.316	4	0	1	2206.258	3	198.41	4
279	7	max	.014	2	.015	2	0	1	1.915e-3	4	NC	4	NC	1
280		min	-.02	3	-.026	3	-.277	4	0	1	2399.308	3	226.362	4
281	8	max	.013	2	.013	2	0	1	1.989e-3	4	NC	1	NC	1
282		min	-.018	3	-.024	3	-.24	4	0	1	2628.546	3	261.842	4
283	9	max	.012	2	.011	2	0	1	2.064e-3	4	NC	1	NC	1
284		min	-.017	3	-.022	3	-.204	4	0	1	2904.929	3	307.848	4
285	10	max	.011	2	.009	2	0	1	2.138e-3	4	NC	1	NC	1
286		min	-.015	3	-.019	3	-.17	4	0	1	3244.289	3	369.058	4
287	11	max	.01	2	.007	2	0	1	2.212e-3	4	NC	1	NC	1
288		min	-.013	3	-.017	3	-.138	4	0	1	3670.383	3	453.104	4
289	12	max	.008	2	.006	2	0	1	2.287e-3	4	NC	1	NC	1
290		min	-.012	3	-.015	3	-.109	4	0	1	4220.556	3	573.145	4
291	13	max	.007	2	.004	2	0	1	2.361e-3	4	NC	1	NC	1
292		min	-.01	3	-.013	3	-.083	4	0	1	4957.064	3	753.513	4
293	14	max	.006	2	.003	2	0	1	2.436e-3	4	NC	1	NC	1
294		min	-.008	3	-.01	3	-.06	4	0	1	5991.992	3	1043.459	4
295	15	max	.005	2	.002	2	0	1	2.51e-3	4	NC	1	NC	1
296		min	-.007	3	-.008	3	-.04	4	0	1	7549.537	3	1555.751	4
297	16	max	.004	2	.001	2	0	1	2.584e-3	4	NC	1	NC	1
298		min	-.005	3	-.006	3	-.024	4	0	1	NC	1	2599.115	4
299	17	max	.002	2	0	2	0	1	2.659e-3	4	NC	1	NC	1
300		min	-.003	3	-.004	3	-.012	4	0	1	NC	1	5300.833	4
301	18	max	.001	2	0	2	0	1	2.733e-3	4	NC	1	NC	1
302		min	-.002	3	-.002	3	-.004	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	2.807e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	0	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-6.849e-4	4	NC	1	NC	1
307	2	max	.001	3	0	2	.013	4	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	-9.884e-5	4	NC	1	NC	1
309	3	max	.003	3	0	2	.026	4	4.872e-4	4	NC	1	NC	1
310		min	-.002	2	-.005	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	.037	4	1.073e-3	4	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.002	15	.048	4	1.659e-3	4	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.006	3	-.002	15	.057	4	2.245e-3	4	NC	1	NC	1
316		min	-.006	2	-.012	3	0	1	0	1	8516.867	3	NC	1
317	7	max	.008	3	-.003	15	.067	4	2.831e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.007	2	-.014	3	0	1	0	1	7614.889	3	NC	1
319	8	max	-.009	3	-.003	15	.075	4	3.417e-3	4	NC	1	NC	1
320		min	-.008	2	-.015	3	0	1	0	1	7082.417	3	NC	1
321	9	max	.01	3	-.003	15	.083	4	4.003e-3	4	NC	1	NC	1
322		min	-.01	2	-.015	3	0	1	0	1	6809.29	3	NC	1
323	10	max	.011	3	-.003	15	.091	4	4.589e-3	4	NC	1	NC	1
324		min	-.011	2	-.016	3	0	1	0	1	6746.271	3	NC	1
325	11	max	.013	3	-.003	15	.099	4	5.175e-3	4	NC	1	NC	1
326		min	-.012	2	-.016	3	0	1	0	1	6881.301	3	NC	1
327	12	max	.014	3	-.003	15	.107	4	5.762e-3	4	NC	1	NC	1
328		min	-.013	2	-.015	3	0	1	0	1	7233.987	3	NC	1
329	13	max	.015	3	-.003	15	.115	4	6.348e-3	4	NC	1	NC	1
330		min	-.014	2	-.015	3	0	1	0	1	7759.414	4	NC	1
331	14	max	.016	3	-.003	15	.123	4	6.934e-3	4	NC	1	NC	1
332		min	-.016	2	-.014	3	0	1	0	1	8650.812	4	NC	1
333	15	max	.018	3	-.002	15	.132	4	7.52e-3	4	NC	1	NC	1
334		min	-.017	2	-.012	3	0	1	0	1	NC	1	NC	1
335	16	max	.019	3	-.002	15	.142	4	8.106e-3	4	NC	1	NC	1
336		min	-.018	2	-.011	3	0	1	0	1	NC	1	NC	1
337	17	max	.02	3	-.001	15	.153	4	8.692e-3	4	NC	1	NC	1
338		min	-.019	2	-.009	3	0	1	0	1	NC	1	NC	1
339	18	max	.021	3	0	15	.164	4	9.278e-3	4	NC	1	NC	1
340		min	-.02	2	-.007	3	0	1	0	1	NC	1	NC	1
341	19	max	.023	3	0	15	.177	4	9.864e-3	4	NC	1	NC	1
342		min	-.022	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	2	.02	2	0	0	1	NC	1	NC	1
344		min	-.002	3	-.023	3	-.177	4	-4.979e-5	5	NC	1	139.739	4
345	2	max	.007	2	.019	2	0	1	0	1	NC	1	NC	1
346		min	-.001	3	-.022	3	-.163	4	-4.979e-5	5	NC	1	151.917	4
347	3	max	.006	2	.018	2	0	1	0	1	NC	1	NC	1
348		min	-.001	3	-.02	3	-.149	4	-4.979e-5	5	NC	1	166.411	4
349	4	max	.006	2	.017	2	0	1	0	1	NC	1	NC	1
350		min	-.001	3	-.019	3	-.135	4	-4.979e-5	5	NC	1	183.824	4
351	5	max	.006	2	.016	2	0	1	0	1	NC	1	NC	1
352		min	-.001	3	-.018	3	-.121	4	-4.979e-5	5	NC	1	204.971	4
353	6	max	.005	2	.015	2	0	1	0	1	NC	1	NC	1
354		min	-.001	3	-.017	3	-.107	4	-4.979e-5	5	NC	1	230.988	4
355	7	max	.005	2	.014	2	0	1	0	1	NC	1	NC	1
356		min	-.001	3	-.015	3	-.094	4	-4.979e-5	5	NC	1	263.482	4
357	8	max	.004	2	.012	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.014	3	-.081	4	-4.979e-5	5	NC	1	304.8	4
359	9	max	.004	2	.011	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.013	3	-.069	4	-4.979e-5	5	NC	1	358.474	4
361	10	max	.004	2	.01	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.012	3	-.058	4	-4.979e-5	5	NC	1	430.025	4
363	11	max	.003	2	.009	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.01	3	-.047	4	-4.979e-5	5	NC	1	528.484	4
365	12	max	.003	2	.008	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.009	3	-.037	4	-4.979e-5	5	NC	1	669.473	4
367	13	max	.002	2	.007	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.008	3	-.028	4	-4.979e-5	5	NC	1	881.983	4
369	14	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.006	3	-.02	4	-4.979e-5	5	NC	1	1224.986	4
371	15	max	.002	2	.005	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.005	3	-.014	4	-4.979e-5	5	NC	1	1834.408	4
373	16	max	.001	2	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.004	3	-.008	4	-4.979e-5	5	NC	1	3085.922	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	2	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.003	3	-.004	4	-4.979e-5	5	NC	1	6372.056	4
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-4.979e-5	5	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-4.979e-5	5	NC	1	NC	1
381	M10	1	max	.007	2	.008	2	0	12	1.471e-3	4	NC	1	NC	2
382			min	-.009	3	-.013	3	-.524	4	1.224e-5	12	8140.005	2	119.703	4
383		2	max	.006	2	.007	2	0	12	1.544e-3	4	NC	1	NC	1
384			min	-.009	3	-.012	3	-.481	4	1.151e-5	12	9299.251	2	130.31	4
385		3	max	.006	2	.006	2	0	12	1.618e-3	4	NC	1	NC	1
386			min	-.008	3	-.012	3	-.439	4	1.078e-5	12	NC	1	142.901	4
387		4	max	.006	2	.005	2	0	12	1.692e-3	4	NC	1	NC	1
388			min	-.008	3	-.011	3	-.397	4	1.006e-5	12	NC	1	157.992	4
389		5	max	.005	2	.004	2	0	12	1.765e-3	4	NC	1	NC	1
390			min	-.007	3	-.011	3	-.356	4	9.331e-6	12	NC	1	176.282	4
391		6	max	.005	2	.003	2	0	12	1.839e-3	4	NC	1	NC	1
392			min	-.007	3	-.01	3	-.316	4	8.605e-6	12	NC	1	198.74	4
393		7	max	.004	2	.002	2	0	12	1.913e-3	4	NC	1	NC	1
394			min	-.006	3	-.009	3	-.277	4	7.878e-6	12	NC	1	226.74	4
395		8	max	.004	2	.002	2	0	12	1.986e-3	4	NC	1	NC	1
396			min	-.006	3	-.009	3	-.239	4	7.152e-6	12	NC	1	262.28	4
397		9	max	.004	2	0	2	0	12	2.06e-3	4	NC	1	NC	1
398			min	-.005	3	-.008	3	-.203	4	6.426e-6	12	NC	1	308.365	4
399		10	max	.003	2	0	2	0	12	2.133e-3	4	NC	1	NC	1
400			min	-.005	3	-.008	3	-.17	4	5.7e-6	12	NC	1	369.681	4
401		11	max	.003	2	0	2	0	12	2.207e-3	4	NC	1	NC	1
402			min	-.004	3	-.007	3	-.138	4	4.974e-6	12	NC	1	453.873	4
403		12	max	.003	2	0	2	0	12	2.281e-3	4	NC	1	NC	1
404			min	-.004	3	-.006	3	-.109	4	4.248e-6	12	NC	1	574.126	4
405		13	max	.002	2	0	2	0	12	2.354e-3	4	NC	1	NC	1
406			min	-.003	3	-.005	3	-.083	4	3.522e-6	12	NC	1	754.818	4
407		14	max	.002	2	-.001	15	0	12	2.428e-3	4	NC	1	NC	1
408			min	-.003	3	-.005	3	-.06	4	2.796e-6	12	NC	1	1045.293	4
409		15	max	.001	2	0	15	0	12	2.502e-3	4	NC	1	NC	1
410			min	-.002	3	-.004	3	-.04	4	2.07e-6	12	NC	1	1558.545	4
411		16	max	.001	2	0	15	0	12	2.575e-3	4	NC	1	NC	1
412			min	-.002	3	-.003	3	-.024	4	1.186e-6	10	NC	1	2603.944	4
413		17	max	0	2	0	15	0	12	2.649e-3	4	NC	1	NC	1
414			min	-.001	3	-.002	3	-.012	4	1.172e-7	10	NC	1	5311.275	4
415		18	max	0	2	0	15	0	12	2.722e-3	4	NC	1	NC	1
416			min	0	3	-.001	4	-.004	4	-7.937e-6	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.796e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.893e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	6.381e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.819e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	-7.686e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-9.242e-5	4	NC	1	NC	1
423		3	max	0	3	0	15	.026	4	4.97e-4	4	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-3.201e-5	1	NC	1	NC	1
425		4	max	.001	3	-.001	15	.037	4	1.086e-3	4	NC	1	NC	1
426			min	0	2	-.006	4	0	1	-5.12e-5	1	NC	1	NC	1
427		5	max	.002	3	-.002	15	.047	4	1.676e-3	4	NC	1	NC	1
428			min	-.001	2	-.008	4	0	1	-7.04e-5	1	NC	1	NC	1
429		6	max	.002	3	-.002	15	.057	4	2.265e-3	4	NC	1	NC	1
430			min	-.002	2	-.01	4	0	1	-8.96e-5	1	9856.747	4	NC	1
431		7	max	.002	3	-.003	15	.066	4	2.855e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.011	4	0	1	-1.088e-4	1	8467.83	4	NC	1
433		8	max	.003	3	-.003	15	.075	4	3.444e-3	4	NC	1	NC	1
434			min	-.002	2	-.012	4	-.001	1	-1.28e-4	1	7611.229	4	NC	1
435		9	max	.003	3	-.003	15	.083	4	4.034e-3	4	NC	2	NC	1
436			min	-.003	2	-.013	4	-.001	1	-1.472e-4	1	7105.801	4	NC	1
437		10	max	.004	3	-.003	15	.091	4	4.623e-3	4	NC	2	NC	1
438			min	-.003	2	-.014	4	-.002	1	-1.664e-4	1	6863.728	4	NC	1
439		11	max	.004	3	-.003	15	.099	4	5.213e-3	4	NC	2	NC	1
440			min	-.003	2	-.014	4	-.002	1	-1.856e-4	1	6849.428	4	NC	1
441		12	max	.004	3	-.003	15	.107	4	5.802e-3	4	NC	2	NC	1
442			min	-.004	2	-.014	4	-.002	1	-2.048e-4	1	7065.901	4	NC	1
443		13	max	.005	3	-.003	15	.115	4	6.392e-3	4	NC	1	NC	1
444			min	-.004	2	-.013	4	-.003	1	-2.24e-4	1	7557.65	4	NC	1
445		14	max	.005	3	-.003	15	.124	4	6.981e-3	4	NC	1	NC	1
446			min	-.004	2	-.012	4	-.003	1	-2.432e-4	1	8433.642	4	NC	1
447		15	max	.006	3	-.003	15	.133	4	7.57e-3	4	NC	1	NC	1
448			min	-.005	2	-.01	4	-.004	1	-2.624e-4	1	9935.019	4	NC	1
449		16	max	.006	3	-.002	15	.143	4	8.16e-3	4	NC	1	NC	1
450			min	-.005	2	-.008	4	-.004	1	-2.815e-4	1	NC	1	NC	1
451		17	max	.006	3	-.002	15	.154	4	8.749e-3	4	NC	1	NC	1
452			min	-.005	2	-.006	4	-.005	1	-3.007e-4	1	NC	1	NC	1
453		18	max	.007	3	-.001	15	.166	4	9.339e-3	4	NC	1	NC	1
454			min	-.006	2	-.004	1	-.006	1	-3.199e-4	1	NC	1	NC	1
455		19	max	.007	3	0	10	.179	4	9.928e-3	4	NC	1	NC	1
456			min	-.006	2	-.002	1	-.006	1	-3.391e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.006	2	.006	1	8.303e-6	5	NC	1	NC	3
458			min	0	3	-.007	3	-.179	4	-5.214e-5	1	NC	1	138.572	4
459		2	max	.002	1	.005	2	.006	1	8.303e-6	5	NC	1	NC	2
460			min	0	3	-.007	3	-.165	4	-5.214e-5	1	NC	1	150.642	4
461		3	max	.002	1	.005	2	.005	1	8.303e-6	5	NC	1	NC	2
462			min	0	3	-.007	3	-.15	4	-5.214e-5	1	NC	1	165.008	4
463		4	max	.002	1	.005	2	.005	1	8.303e-6	5	NC	1	NC	2
464			min	0	3	-.006	3	-.136	4	-5.214e-5	1	NC	1	182.268	4
465		5	max	.002	1	.004	2	.004	1	8.303e-6	5	NC	1	NC	2
466			min	0	3	-.006	3	-.122	4	-5.214e-5	1	NC	1	203.229	4
467		6	max	.002	1	.004	2	.004	1	8.303e-6	5	NC	1	NC	2
468			min	0	3	-.005	3	-.108	4	-5.214e-5	1	NC	1	229.018	4
469		7	max	.002	1	.004	2	.003	1	8.303e-6	5	NC	1	NC	2
470			min	0	3	-.005	3	-.095	4	-5.214e-5	1	NC	1	261.227	4
471		8	max	.002	1	.003	2	.003	1	8.303e-6	5	NC	1	NC	2
472			min	0	3	-.005	3	-.082	4	-5.214e-5	1	NC	1	302.182	4
473		9	max	.001	1	.003	2	.002	1	8.303e-6	5	NC	1	NC	2
474			min	0	3	-.004	3	-.07	4	-5.214e-5	1	NC	1	355.386	4
475		10	max	.001	1	.003	2	.002	1	8.303e-6	5	NC	1	NC	1
476			min	0	3	-.004	3	-.058	4	-5.214e-5	1	NC	1	426.309	4
477		11	max	.001	1	.002	2	.002	1	8.303e-6	5	NC	1	NC	1
478			min	0	3	-.003	3	-.047	4	-5.214e-5	1	NC	1	523.904	4
479		12	max	.001	1	.002	2	.001	1	8.303e-6	5	NC	1	NC	1
480			min	0	3	-.003	3	-.037	4	-5.214e-5	1	NC	1	663.655	4
481		13	max	0	1	.002	2	.001	1	8.303e-6	5	NC	1	NC	1
482			min	0	3	-.002	3	-.028	4	-5.214e-5	1	NC	1	874.297	4
483		14	max	0	1	.002	2	0	1	8.303e-6	5	NC	1	NC	1
484			min	0	3	-.002	3	-.02	4	-5.214e-5	1	NC	1	1214.283	4
485		15	max	0	1	.001	2	0	1	8.303e-6	5	NC	1	NC	1
486			min	0	3	-.002	3	-.014	4	-5.214e-5	1	NC	1	1818.336	4
487		16	max	0	1	0	2	0	1	8.303e-6	5	NC	1	NC	1
488			min	0	3	-.001	3	-.008	4	-5.214e-5	1	NC	1	3058.808	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	8.303e-6	5	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-5.214e-5	1	NC	1	6315.885	4
491		18	max	0	1	0	2	0	1	8.303e-6	5	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-5.214e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	8.303e-6	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-5.214e-5	1	NC	1	NC	1
495	M1	1	max	.009	3	.123	2	.553	4	1.236e-2	2	NC	1	NC	1
496			min	-.005	2	-.022	3	0	12	-2.505e-2	3	NC	1	NC	1
497		2	max	.009	3	.059	2	.537	4	7.018e-3	4	NC	4	NC	1
498			min	-.005	2	-.008	3	-.005	1	-1.239e-2	3	1782.838	2	NC	1
499		3	max	.009	3	.014	3	.52	4	1.189e-2	4	NC	5	NC	1
500			min	-.005	2	-.011	2	-.007	1	-1.284e-4	3	859.658	2	7646.456	5
501		4	max	.009	3	.052	3	.503	4	1.032e-2	4	NC	5	NC	1
502			min	-.005	2	-.089	2	-.006	1	-4.802e-3	3	543.043	2	5463.307	5
503		5	max	.009	3	.101	3	.485	4	8.748e-3	4	NC	5	NC	1
504			min	-.005	2	-.17	2	-.004	1	-9.475e-3	3	392.151	2	4362.787	5
505		6	max	.008	3	.154	3	.467	4	1.29e-2	2	NC	15	NC	1
506			min	-.005	2	-.249	2	-.002	1	-1.415e-2	3	308.995	2	3698.362	5
507		7	max	.008	3	.205	3	.449	4	1.719e-2	2	NC	15	NC	1
508			min	-.005	2	-.32	2	0	3	-1.882e-2	3	259.893	2	3234.608	4
509		8	max	.008	3	.248	3	.429	4	2.149e-2	2	9763.063	15	NC	1
510			min	-.005	2	-.375	2	0	12	-2.349e-2	3	230.842	2	2894.326	4
511		9	max	.008	3	.276	3	.409	4	2.435e-2	2	9129.703	15	NC	1
512			min	-.004	2	-.411	2	0	1	-2.367e-2	3	215.716	2	2676.83	4
513		10	max	.008	3	.286	3	.387	4	2.625e-2	2	8936.579	15	NC	1
514			min	-.004	2	-.423	2	0	12	-2.085e-2	3	211.277	2	2608.244	4
515		11	max	.008	3	.279	3	.363	4	2.815e-2	2	9129.34	15	NC	1
516			min	-.004	2	-.411	2	0	12	-1.803e-2	3	216.428	2	2657.216	4
517		12	max	.007	3	.256	3	.337	4	2.715e-2	2	9762.231	15	NC	1
518			min	-.004	2	-.374	2	0	1	-1.513e-2	3	233.003	2	2833.902	4
519		13	max	.007	3	.218	3	.308	4	2.177e-2	2	NC	15	NC	1
520			min	-.004	2	-.315	2	0	1	-1.211e-2	3	265.139	2	3314.338	4
521		14	max	.007	3	.17	3	.277	4	1.639e-2	2	NC	15	NC	1
522			min	-.004	2	-.242	2	0	12	-9.095e-3	3	320.177	2	4333.502	4
523		15	max	.007	3	.115	3	.245	4	1.102e-2	2	NC	5	NC	1
524			min	-.004	2	-.162	2	0	12	-6.076e-3	3	415.106	2	6567.933	4
525		16	max	.007	3	.059	3	.213	4	8.397e-3	4	NC	5	NC	1
526			min	-.004	2	-.08	2	0	12	-3.057e-3	3	591.303	2	NC	1
527		17	max	.007	3	.005	3	.183	4	9.533e-3	4	NC	5	NC	1
528			min	-.004	2	-.006	2	0	12	-3.808e-5	3	968.755	2	NC	1
529		18	max	.007	3	.055	2	.157	4	9.383e-3	2	NC	4	NC	1
530			min	-.004	2	-.044	3	0	12	-3.857e-3	3	2060.07	2	NC	1
531		19	max	.007	3	.11	2	.134	4	1.884e-2	2	NC	1	NC	1
532			min	-.004	2	-.089	3	0	1	-7.835e-3	3	NC	1	NC	1
533	M5	1	max	.028	3	.271	2	.553	4	0	1	NC	1	NC	1
534			min	-.019	2	-.013	3	0	1	-5.131e-6	4	NC	1	NC	1
535		2	max	.028	3	.128	2	.54	4	6.1e-3	4	NC	5	NC	1
536			min	-.019	2	0	3	0	1	0	1	806.883	2	NC	1
537		3	max	.028	3	.043	3	.524	4	1.202e-2	4	NC	5	NC	1
538			min	-.019	2	-.033	2	0	1	0	1	379.561	2	6252.141	4
539		4	max	.027	3	.139	3	.507	4	9.79e-3	4	NC	15	NC	1
540			min	-.019	2	-.226	2	0	1	0	1	232.266	2	4782.266	4
541		5	max	.027	3	.273	3	.488	4	7.564e-3	4	7846.725	15	NC	1
542			min	-.019	2	-.435	2	0	1	0	1	163.45	2	4062.918	4
543		6	max	.026	3	.423	3	.468	4	5.337e-3	4	6036.753	15	NC	1
544			min	-.018	2	-.642	2	0	1	0	1	126.324	2	3616.544	4
545		7	max	.026	3	.57	3	.448	4	3.111e-3	4	4992.4	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.018	2	-.83	2	0	1	0	1	104.783	2	3269.704	4
547	8	max	.025	3	.694	3	.429	4	8.851e-4	4	4385.558	15	NC	1
548		min	-.018	2	-.981	2	0	1	0	1	92.217	2	2942.476	4
549	9	max	.025	3	.774	3	.41	4	0	1	4074.51	15	NC	1
550		min	-.017	2	-1.076	2	0	1	-3.511e-6	5	85.759	2	2671.781	4
551	10	max	.024	3	.802	3	.387	4	0	1	3980.825	15	NC	1
552		min	-.017	2	-1.108	2	0	1	-3.389e-6	5	83.868	2	2626.42	4
553	11	max	.024	3	.782	3	.363	4	0	1	4074.658	15	NC	1
554		min	-.017	2	-1.076	2	0	1	-3.267e-6	5	86.057	2	2688.264	4
555	12	max	.023	3	.715	3	.338	4	6.791e-4	4	4385.905	15	NC	1
556		min	-.017	2	-.977	2	0	1	0	1	93.195	2	2784.01	4
557	13	max	.022	3	.606	3	.309	4	2.388e-3	4	4993.096	15	NC	1
558		min	-.016	2	-.818	2	0	1	0	1	107.322	2	3255.692	4
559	14	max	.022	3	.468	3	.276	4	4.097e-3	4	6038.098	15	NC	1
560		min	-.016	2	-.621	2	0	1	0	1	132.054	2	4492.593	4
561	15	max	.021	3	.315	3	.242	4	5.805e-3	4	7849.361	15	NC	1
562		min	-.016	2	-.408	2	0	1	0	1	175.956	2	7975.884	4
563	16	max	.021	3	.16	3	.208	4	7.514e-3	4	NC	15	NC	1
564		min	-.016	2	-.2	2	0	1	0	1	260.567	2	NC	1
565	17	max	.02	3	.014	3	.178	4	9.223e-3	4	NC	5	NC	1
566		min	-.015	2	-.019	2	0	1	0	1	449.435	2	NC	1
567	18	max	.02	3	.118	2	.153	4	4.683e-3	4	NC	5	NC	1
568		min	-.015	2	-.11	3	0	1	0	1	996.474	2	NC	1
569	19	max	.02	3	.23	2	.135	4	0	1	NC	1	NC	1
570		min	-.015	2	-.222	3	0	1	-2.845e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.123	.552	4	2.505e-2	3	NC	1	NC	1
572		min	-.005	2	-.022	3	0	1	-1.236e-2	2	NC	1	NC	1
573	2	max	.009	3	.059	2	.539	4	1.239e-2	3	NC	4	NC	1
574		min	-.005	2	-.008	3	0	12	-6.067e-3	2	1782.838	2	NC	1
575	3	max	.009	3	.014	3	.524	4	1.198e-2	4	NC	5	NC	1
576		min	-.005	2	-.011	2	0	12	-3.408e-5	10	859.658	2	6480.88	4
577	4	max	.009	3	.052	3	.506	4	9.454e-3	5	NC	5	NC	1
578		min	-.005	2	-.089	2	0	12	-4.312e-3	2	543.043	2	4874.973	4
579	5	max	.009	3	.101	3	.488	4	9.475e-3	3	NC	5	NC	1
580		min	-.005	2	-.17	2	0	12	-8.606e-3	2	392.151	2	4079.215	4
581	6	max	.008	3	.154	3	.468	4	1.415e-2	3	NC	15	NC	1
582		min	-.005	2	-.249	2	0	12	-1.29e-2	2	308.995	2	3590.503	4
583	7	max	.008	3	.205	3	.448	4	1.882e-2	3	NC	15	NC	1
584		min	-.005	2	-.32	2	0	1	-1.719e-2	2	259.893	2	3231.016	4
585	8	max	.008	3	.248	3	.429	4	2.349e-2	3	9742.094	15	NC	1
586		min	-.005	2	-.375	2	0	1	-2.149e-2	2	230.842	2	2919.635	4
587	9	max	.008	3	.276	3	.409	4	2.367e-2	3	9110.322	15	NC	1
588		min	-.004	2	-.411	2	0	12	-2.435e-2	2	215.716	2	2669.494	4
589	10	max	.008	3	.286	3	.387	4	2.085e-2	3	8917.656	15	NC	1
590		min	-.004	2	-.423	2	0	1	-2.625e-2	2	211.277	2	2609.366	4
591	11	max	.008	3	.279	3	.363	4	1.803e-2	3	9109.944	15	NC	1
592		min	-.004	2	-.411	2	0	1	-2.815e-2	2	216.428	2	2666.255	4
593	12	max	.007	3	.256	3	.338	4	1.513e-2	3	9741.345	15	NC	1
594		min	-.004	2	-.374	2	0	12	-2.715e-2	2	233.003	2	2810.079	4
595	13	max	.007	3	.218	3	.308	4	1.211e-2	3	NC	15	NC	1
596		min	-.004	2	-.315	2	0	10	-2.177e-2	2	265.139	2	3313.906	4
597	14	max	.007	3	.17	3	.276	4	9.095e-3	3	NC	15	NC	1
598		min	-.004	2	-.242	2	-.002	1	-1.639e-2	2	320.177	2	4467.386	5
599	15	max	.007	3	.115	3	.242	4	6.076e-3	3	NC	5	NC	1
600		min	-.004	2	-.162	2	-.004	1	-1.102e-2	2	415.106	2	7225.198	5
601	16	max	.007	3	.059	3	.21	4	7.406e-3	5	NC	5	NC	1
602		min	-.004	2	-.08	2	-.006	1	-5.638e-3	2	591.303	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.007	3	.005	3	.18	4	9.302e-3	4	NC	5	NC	1
604		min	-.004	2	-.006	2	-.006	1	-4.556e-4	1	968.755	2	NC	1
605	18	max	.007	3	.055	2	.155	4	4.487e-3	5	NC	4	NC	1
606		min	-.004	2	-.044	3	-.004	1	-9.383e-3	2	2060.07	2	NC	1
607	19	max	.007	3	.11	2	.135	4	7.835e-3	3	NC	1	NC	1
608		min	-.004	2	-.089	3	0	12	-1.884e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



**Anchor Designer™**  
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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 37-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

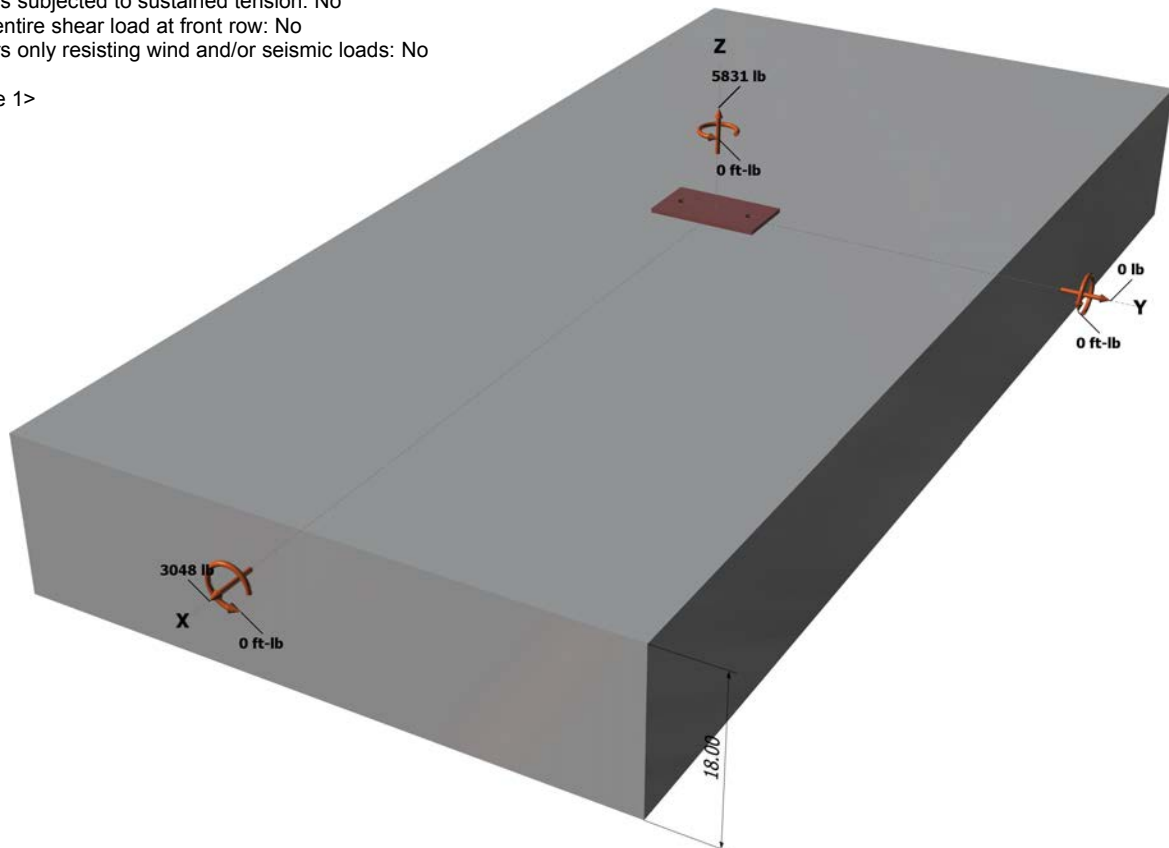
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

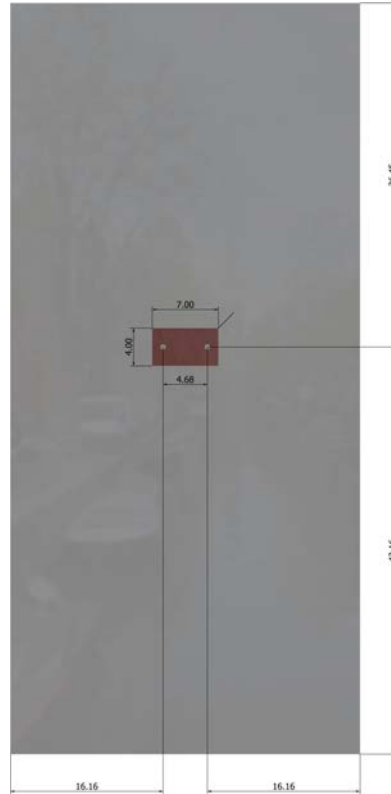
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Address:			
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

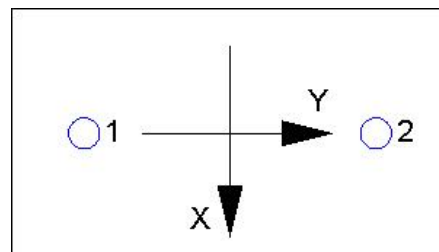
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2915.5	1524.0	0.0	1524.0
2	2915.5	1524.0	0.0	1524.0
Sum	5831.0	3048.0	0.0	3048.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5831  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
666.00	648.00	1.000	0.969	1.000	1.000	15593	0.70	10875

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	16.16	24369

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
872.64	1175.16	1.000	1.000	1.000	24369	0.70	25334

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2916	6071	0.48	Pass	
Concrete breakout	5831	10231	0.57	Pass	
<b>Adhesive</b>	<b>5831</b>	<b>8093</b>	<b>0.72</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1524</b>	<b>3156</b>	<b>0.48</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3048	10875	0.28	Pass	
Concrete breakout y-	1524	25334	0.06	Pass	
Pryout	3048	20601	0.15	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.72	0.48	120.3 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.