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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

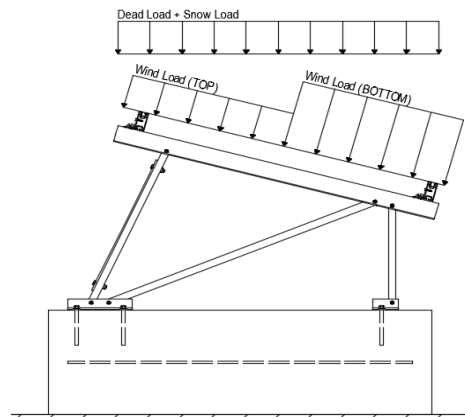
### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	115 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 20.76 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

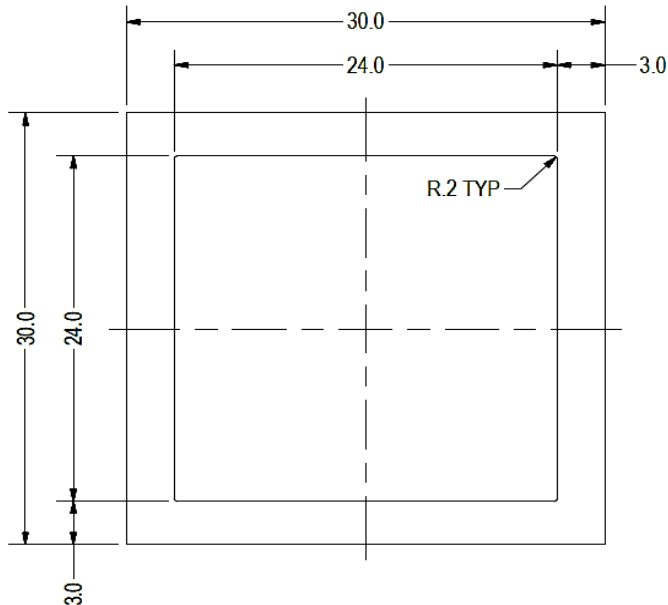
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	1.708 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>14%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.164 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>4%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

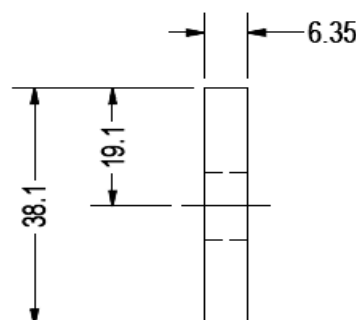
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.369 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<u>17%</u>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.006 k-ft
$P_n$ =	0.028 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>13%</u>



A cross brace kit is required every 14 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

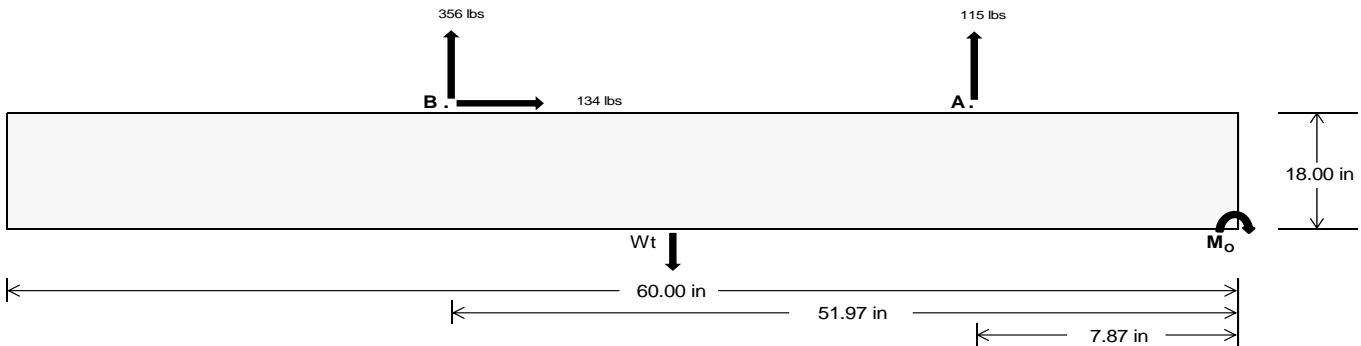
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>507.41</u>	<u>1550.73</u>	k
Compressive Load =	<u>2220.11</u>	<u>1626.70</u>	k
Lateral Load =	<u>3.84</u>	<u>582.19</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21838.0$  in-lbs  
Resisting Force Required = 727.93 lbs  
S.F. = 1.67  
Weight Required = 1213.22 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 134.26 lbs  
Friction = 0.4  
Weight Required = 335.64 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 134.26 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	834 lbs	834 lbs	834 lbs	834 lbs	618 lbs	618 lbs	618 lbs	618 lbs	1032 lbs	1032 lbs	1032 lbs	1032 lbs	-230 lbs	-230 lbs	-230 lbs	-230 lbs
$F_B$	613 lbs	613 lbs	613 lbs	613 lbs	452 lbs	452 lbs	452 lbs	452 lbs	755 lbs	755 lbs	755 lbs	755 lbs	-713 lbs	-713 lbs	-713 lbs	-713 lbs
$F_V$	58 lbs	58 lbs	58 lbs	58 lbs	240 lbs	240 lbs	240 lbs	240 lbs	219 lbs	219 lbs	219 lbs	219 lbs	-269 lbs	-269 lbs	-269 lbs	-269 lbs
$P_{total}$	3350 lbs	3440 lbs	3531 lbs	3622 lbs	2973 lbs	3064 lbs	3154 lbs	3245 lbs	3690 lbs	3781 lbs	3872 lbs	3962 lbs	199 lbs	254 lbs	308 lbs	362 lbs
$M$	502 lbs-ft	502 lbs-ft	502 lbs-ft	502 lbs-ft	672 lbs-ft	672 lbs-ft	672 lbs-ft	672 lbs-ft	849 lbs-ft	849 lbs-ft	849 lbs-ft	849 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft
$e$	0.15 ft	0.15 ft	0.14 ft	0.14 ft	0.23 ft	0.22 ft	0.21 ft	0.21 ft	0.23 ft	0.22 ft	0.22 ft	0.21 ft	2.40 ft	1.88 ft	1.55 ft	1.32 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	314.1 psf	309.7 psf	305.7 psf	302.0 psf	247.6 psf	246.2 psf	245.0 psf	243.8 psf	305.3 psf	301.3 psf	297.7 psf	294.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	451.6 psf	441.0 psf	431.3 psf	422.4 psf	431.9 psf	422.2 psf	413.3 psf	405.1 psf	538.2 psf	523.6 psf	510.3 psf	498.1 psf	736.5 psf	149.6 psf	112.9 psf	102.2 psf

Maximum Bearing Pressure = 737 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

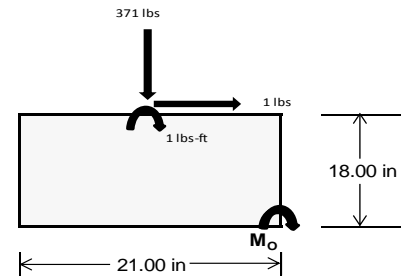
## Overturning Check

$M_o = 322.5 \text{ ft-lbs}$   
 Resisting Force Required = 368.58 lbs  
 S.F. = 1.67  
 Weight Required = 614.30 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	88 lbs	248 lbs	84 lbs	376 lbs	1171 lbs	371 lbs	26 lbs	73 lbs	24 lbs
$F_v$	3 lbs	3 lbs	0 lbs	15 lbs	14 lbs	1 lbs	1 lbs	1 lbs	0 lbs
$P_{total}$	2444 lbs	2604 lbs	2440 lbs	2619 lbs	3414 lbs	2614 lbs	715 lbs	761 lbs	713 lbs
$M$	4 lbs-ft	4 lbs-ft	0 lbs-ft	26 lbs-ft	22 lbs-ft	2 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.73 ft	1.74 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
$f_{min}$	277.6 sqft	296.0 sqft	278.7 sqft	289.2 sqft	381.7 sqft	297.9 sqft	81.2 sqft	86.6 sqft	81.5 sqft
$f_{max}$	281.1 psf	299.2 psf	278.9 psf	309.4 psf	398.6 psf	299.6 psf	82.2 psf	87.5 psf	81.6 psf



Maximum Bearing Pressure = 399 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.**

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.392 k
Allowable Uplift =	1.214 k
Utilization =	<u>32%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.708 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>30%</u>

#### Diagonal Strut

Maximum Axial Load =	0.164 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.369 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>24%</u>

#### Bracing

Maximum Axial Load =	0.028 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.042 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 96.00 \text{ in}$$

$$J = 0.427$$

$$200.222$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 96.00 \text{ in}$$

$$J = 0.427$$

$$217.57$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.787 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$C_c = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	101.643	2	349.892	1	.031	2	0	1	0	1	0	1
2		min	-140.361	3	-362.743	3	-.108	3	0	3	0	1	0	1
3	N7	max	0	15	572.28	1	-.047	15	0	15	0	1	0	1
4		min	-.184	1	-111.371	3	-1.295	1	-.002	1	0	1	0	1
5	N15	max	0	15	1707.776	1	.493	1	.001	1	0	1	0	1
6		min	-1.935	1	-390.312	3	-.233	3	0	3	0	1	0	1
7	N16	max	421.679	2	1251.305	1	-.164	10	0	1	0	1	0	1
8		min	-447.839	3	-1192.873	3	-26.235	3	0	3	0	1	0	1
9	N23	max	0	15	572.176	1	2.956	1	.005	1	0	1	0	1
10		min	-.184	1	-110.974	3	.099	15	0	15	0	1	0	1
11	N24	max	102.042	2	355.229	1	26.461	3	.002	1	0	1	0	1
12		min	-140.421	3	-359.862	3	.034	10	0	3	0	1	0	1
13	Totals:	max	623.468	2	4808.658	1	0	1						
14		min	-728.979	3	-2528.135	3	0	2						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	416.972	1	.661	4	.813	1	0	15	0	3	0	1
2			min	-364.692	3	.157	15	-.057	3	0	1	0	2	0	1
3		2	max	417.068	1	.623	4	.813	1	0	15	0	1	0	15
4			min	-364.62	3	.148	15	-.057	3	0	1	0	10	0	4
5		3	max	417.164	1	.585	4	.813	1	0	15	0	1	0	15
6			min	-364.547	3	.139	15	-.057	3	0	1	0	15	0	4
7		4	max	417.261	1	.547	4	.813	1	0	15	0	1	0	15
8			min	-364.475	3	.13	15	-.057	3	0	1	0	12	0	4
9		5	max	417.357	1	.509	4	.813	1	0	15	0	1	0	15
10			min	-364.403	3	.121	15	-.057	3	0	1	0	3	0	4
11		6	max	417.453	1	.471	4	.813	1	0	15	0	1	0	15
12			min	-364.331	3	.113	15	-.057	3	0	1	0	3	0	4
13		7	max	417.55	1	.434	4	.813	1	0	15	0	1	0	15
14			min	-364.258	3	.104	15	-.057	3	0	1	0	3	0	4
15		8	max	417.646	1	.396	4	.813	1	0	15	0	1	0	15
16			min	-364.186	3	.095	15	-.057	3	0	1	0	3	0	4
17		9	max	417.742	1	.358	4	.813	1	0	15	0	1	0	15
18			min	-364.114	3	.086	15	-.057	3	0	1	0	3	0	4
19		10	max	417.839	1	.32	4	.813	1	0	15	.001	1	0	15
20			min	-364.041	3	.077	15	-.057	3	0	1	0	3	0	4
21		11	max	417.935	1	.282	4	.813	1	0	15	.001	1	0	15
22			min	-363.969	3	.068	15	-.057	3	0	1	0	3	0	4
23		12	max	418.032	1	.244	4	.813	1	0	15	.001	1	0	15
24			min	-363.897	3	.059	15	-.057	3	0	1	0	3	0	4
25		13	max	418.128	1	.207	4	.813	1	0	15	.001	1	0	15
26			min	-363.825	3	.05	15	-.057	3	0	1	0	3	0	4
27		14	max	418.224	1	.169	4	.813	1	0	15	.001	1	0	15
28			min	-363.752	3	.041	15	-.057	3	0	1	0	3	0	4
29		15	max	418.321	1	.131	4	.813	1	0	15	.002	1	0	15
30			min	-363.68	3	.032	15	-.057	3	0	1	0	3	0	4
31		16	max	418.417	1	.093	4	.813	1	0	15	.002	1	0	15
32			min	-363.608	3	.024	15	-.057	3	0	1	0	3	0	4
33		17	max	418.513	1	.055	4	.813	1	0	15	.002	1	0	15
34			min	-363.536	3	.002	1	-.057	3	0	1	0	3	0	4
35		18	max	418.61	1	.029	10	.813	1	0	15	.002	1	0	15
36			min	-363.463	3	-.028	1	-.057	3	0	1	0	3	0	4
37		19	max	418.706	1	.004	10	.813	1	0	15	.002	1	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-363.391	3	-.057	1	-.057	3	0	1	0	3	0	4
39	M3	1	max	34.712	10	1.811	4	-.021	15	0	15	.002	1	0	4
40			min	-118.878	1	.427	15	-.688	1	0	1	0	15	0	15
41		2	max	34.656	10	1.633	4	-.021	15	0	15	.002	1	0	4
42			min	-118.945	1	.385	15	-.688	1	0	1	0	15	0	15
43		3	max	34.6	10	1.455	4	-.021	15	0	15	.002	1	0	10
44			min	-119.013	1	.343	15	-.688	1	0	1	0	15	0	1
45		4	max	34.544	10	1.277	4	-.021	15	0	15	.002	1	0	15
46			min	-119.08	1	.301	15	-.688	1	0	1	0	15	0	1
47		5	max	34.488	10	1.099	4	-.021	15	0	15	.001	1	0	15
48			min	-119.147	1	.26	15	-.688	1	0	1	0	15	0	4
49		6	max	34.432	10	.921	4	-.021	15	0	15	.001	1	0	15
50			min	-119.214	1	.218	15	-.688	1	0	1	0	15	0	4
51		7	max	34.377	10	.743	4	-.021	15	0	15	.001	1	0	15
52			min	-119.281	1	.176	15	-.688	1	0	1	0	15	0	4
53		8	max	34.321	10	.565	4	-.021	15	0	15	0	1	0	15
54			min	-119.348	1	.134	15	-.688	1	0	1	0	15	0	4
55		9	max	34.265	10	.387	4	-.021	15	0	15	0	1	0	15
56			min	-119.415	1	.092	15	-.688	1	0	1	0	15	-.001	4
57		10	max	34.209	10	.209	4	-.021	15	0	15	0	1	0	15
58			min	-119.482	1	.05	15	-.688	1	0	1	0	15	-.001	4
59		11	max	34.153	10	.032	10	-.021	15	0	15	0	1	0	15
60			min	-119.549	1	-.003	1	-.688	1	0	1	0	15	-.001	4
61		12	max	34.097	10	-.033	15	-.021	15	0	15	0	1	0	15
62			min	-119.616	1	-.147	4	-.688	1	0	1	0	12	-.001	4
63		13	max	34.041	10	-.075	15	-.021	15	0	15	0	1	0	15
64			min	-119.683	1	-.325	4	-.688	1	0	1	0	12	-.001	4
65		14	max	33.985	10	-.117	15	-.021	15	0	15	0	1	0	15
66			min	-119.751	1	-.503	4	-.688	1	0	1	0	3	-.001	4
67		15	max	33.929	10	-.159	15	-.021	15	0	15	0	15	0	15
68			min	-119.818	1	-.681	4	-.688	1	0	1	0	1	0	4
69		16	max	33.873	10	-.201	15	-.021	15	0	15	0	15	0	15
70			min	-119.885	1	-.859	4	-.688	1	0	1	0	1	0	4
71		17	max	33.817	10	-.243	15	-.021	15	0	15	0	15	0	15
72			min	-119.952	1	-1.037	4	-.688	1	0	1	0	1	0	4
73		18	max	33.762	10	-.284	15	-.021	15	0	15	0	15	0	15
74			min	-120.019	1	-1.215	4	-.688	1	0	1	0	1	0	4
75		19	max	33.706	10	-.326	15	-.021	15	0	15	0	15	0	1
76			min	-120.086	1	-1.393	4	-.688	1	0	1	0	1	0	1
77	M4	1	max	571.115	1	0	1	-.047	15	0	1	0	3	0	1
78			min	-112.245	3	0	1	-1.432	1	0	1	0	1	0	1
79		2	max	571.18	1	0	1	-.047	15	0	1	0	12	0	1
80			min	-112.196	3	0	1	-1.432	1	0	1	0	1	0	1
81		3	max	571.244	1	0	1	-.047	15	0	1	0	15	0	1
82			min	-112.148	3	0	1	-1.432	1	0	1	0	1	0	1
83		4	max	571.309	1	0	1	-.047	15	0	1	0	15	0	1
84			min	-112.099	3	0	1	-1.432	1	0	1	0	1	0	1
85		5	max	571.374	1	0	1	-.047	15	0	1	0	15	0	1
86			min	-112.051	3	0	1	-1.432	1	0	1	0	1	0	1
87		6	max	571.438	1	0	1	-.047	15	0	1	0	15	0	1
88			min	-112.002	3	0	1	-1.432	1	0	1	0	1	0	1
89		7	max	571.503	1	0	1	-.047	15	0	1	0	15	0	1
90			min	-111.954	3	0	1	-1.432	1	0	1	0	1	0	1
91		8	max	571.568	1	0	1	-.047	15	0	1	0	15	0	1
92			min	-111.905	3	0	1	-1.432	1	0	1	0	1	0	1
93		9	max	571.633	1	0	1	-.047	15	0	1	0	15	0	1
94			min	-111.857	3	0	1	-1.432	1	0	1	-.001	1	0	1



Company : Schletter, Inc.  
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Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	571.697	1	0	1	-.047	15	0	1	0	15	0	1
96			min	-111.808	3	0	1	-1.432	1	0	1	-.001	1	0	1
97		11	max	571.762	1	0	1	-.047	15	0	1	0	15	0	1
98			min	-111.759	3	0	1	-1.432	1	0	1	-.001	1	0	1
99		12	max	571.827	1	0	1	-.047	15	0	1	0	15	0	1
100			min	-111.711	3	0	1	-1.432	1	0	1	-.001	1	0	1
101		13	max	571.891	1	0	1	-.047	15	0	1	0	15	0	1
102			min	-111.662	3	0	1	-1.432	1	0	1	-.002	1	0	1
103		14	max	571.956	1	0	1	-.047	15	0	1	0	15	0	1
104			min	-111.614	3	0	1	-1.432	1	0	1	-.002	1	0	1
105		15	max	572.021	1	0	1	-.047	15	0	1	0	15	0	1
106			min	-111.565	3	0	1	-1.432	1	0	1	-.002	1	0	1
107		16	max	572.086	1	0	1	-.047	15	0	1	0	15	0	1
108			min	-111.517	3	0	1	-1.432	1	0	1	-.002	1	0	1
109		17	max	572.15	1	0	1	-.047	15	0	1	0	15	0	1
110			min	-111.468	3	0	1	-1.432	1	0	1	-.002	1	0	1
111		18	max	572.215	1	0	1	-.047	15	0	1	0	15	0	1
112			min	-111.42	3	0	1	-1.432	1	0	1	-.002	1	0	1
113		19	max	572.28	1	0	1	-.047	15	0	1	0	15	0	1
114			min	-111.371	3	0	1	-1.432	1	0	1	-.002	1	0	1
115	M6	1	max	1367.566	1	.643	4	.326	1	0	1	0	3	0	1
116			min	-1195.381	3	.155	15	-.124	3	0	15	0	1	0	1
117		2	max	1367.662	1	.605	4	.326	1	0	1	0	3	0	15
118			min	-1195.309	3	.146	15	-.124	3	0	15	0	1	0	4
119		3	max	1367.759	1	.568	4	.326	1	0	1	0	2	0	15
120			min	-1195.236	3	.137	15	-.124	3	0	15	0	15	0	4
121		4	max	1367.855	1	.53	4	.326	1	0	1	0	1	0	15
122			min	-1195.164	3	.128	15	-.124	3	0	15	0	3	0	4
123		5	max	1367.951	1	.492	4	.326	1	0	1	0	1	0	15
124			min	-1195.092	3	.119	15	-.124	3	0	15	0	3	0	4
125		6	max	1368.048	1	.454	4	.326	1	0	1	0	1	0	15
126			min	-1195.019	3	.11	15	-.124	3	0	15	0	3	0	4
127		7	max	1368.144	1	.416	4	.326	1	0	1	0	1	0	15
128			min	-1194.947	3	.101	15	-.124	3	0	15	0	3	0	4
129		8	max	1368.24	1	.378	4	.326	1	0	1	0	1	0	15
130			min	-1194.875	3	.092	15	-.124	3	0	15	0	3	0	4
131		9	max	1368.337	1	.341	4	.326	1	0	1	0	1	0	15
132			min	-1194.803	3	.084	15	-.124	3	0	15	0	3	0	4
133		10	max	1368.433	1	.303	4	.326	1	0	1	0	1	0	15
134			min	-1194.73	3	.075	15	-.124	3	0	15	0	3	0	4
135		11	max	1368.53	1	.265	4	.326	1	0	1	0	1	0	15
136			min	-1194.658	3	.066	15	-.124	3	0	15	0	3	0	4
137		12	max	1368.626	1	.227	4	.326	1	0	1	0	1	0	15
138			min	-1194.586	3	.057	15	-.124	3	0	15	0	3	0	4
139		13	max	1368.722	1	.189	4	.326	1	0	1	0	1	0	15
140			min	-1194.514	3	.048	15	-.124	3	0	15	0	3	0	4
141		14	max	1368.819	1	.154	2	.326	1	0	1	0	1	0	15
142			min	-1194.441	3	.035	9	-.124	3	0	15	0	3	0	4
143		15	max	1368.915	1	.125	2	.326	1	0	1	0	1	0	15
144			min	-1194.369	3	.01	9	-.124	3	0	15	0	3	0	4
145		16	max	1369.011	1	.095	2	.326	1	0	1	0	1	0	15
146			min	-1194.297	3	-.016	1	-.124	3	0	15	0	3	0	4
147		17	max	1369.108	1	.071	10	.326	1	0	1	0	1	0	15
148			min	-1194.224	3	-.045	1	-.124	3	0	15	0	3	0	4
149		18	max	1369.204	1	.046	10	.326	1	0	1	0	1	0	15
150			min	-1194.152	3	-.074	1	-.124	3	0	15	0	3	0	4
151		19	max	1369.3	1	.022	10	.326	1	0	1	0	1	0	15



***Envelope Member Section Forces (Continued)***

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152			min	-1194.08	3	-.104	1	-.124	3	0	15	0	3	0	4
153	M7	1	max	163.702	2	1.804	4	.014	1	0	2	0	2	0	4
154			min	-167.322	9	.426	15	-.007	3	0	3	0	3	0	15
155		2	max	163.635	2	1.626	4	.014	1	0	2	0	2	0	2
156			min	-167.378	9	.384	15	-.007	3	0	3	0	3	0	15
157		3	max	163.568	2	1.448	4	.014	1	0	2	0	2	0	2
158			min	-167.433	9	.342	15	-.007	3	0	3	0	3	0	9
159		4	max	163.501	2	1.27	4	.014	1	0	2	0	2	0	10
160			min	-167.489	9	.3	15	-.007	3	0	3	0	3	0	1
161		5	max	163.434	2	1.092	4	.014	1	0	2	0	2	0	15
162			min	-167.545	9	.259	15	-.007	3	0	3	0	3	0	1
163		6	max	163.367	2	.914	4	.014	1	0	2	0	2	0	15
164			min	-167.601	9	.217	15	-.007	3	0	3	0	3	0	4
165		7	max	163.3	2	.736	4	.014	1	0	2	0	2	0	15
166			min	-167.657	9	.175	15	-.007	3	0	3	0	3	0	4
167		8	max	163.233	2	.558	4	.014	1	0	2	0	2	0	15
168			min	-167.713	9	.133	15	-.007	3	0	3	0	3	0	4
169		9	max	163.166	2	.38	4	.014	1	0	2	0	2	0	15
170			min	-167.769	9	.091	15	-.007	3	0	3	0	3	-.001	4
171		10	max	163.099	2	.202	4	.014	1	0	2	0	2	0	15
172			min	-167.825	9	.049	15	-.007	3	0	3	0	3	-.001	4
173		11	max	163.031	2	.053	2	.014	1	0	2	0	2	0	15
174			min	-167.881	9	-.02	9	-.007	3	0	3	0	3	-.001	4
175		12	max	162.964	2	-.034	15	.014	1	0	2	0	2	0	15
176			min	-167.937	9	-.155	1	-.007	3	0	3	0	3	-.001	4
177		13	max	162.897	2	-.076	15	.014	1	0	2	0	2	0	15
178			min	-167.993	9	-.332	4	-.007	3	0	3	0	3	-.001	4
179		14	max	162.83	2	-.118	15	.014	1	0	2	0	2	0	15
180			min	-168.049	9	-.51	4	-.007	3	0	3	0	3	-.001	4
181		15	max	162.763	2	-.16	15	.014	1	0	2	0	2	0	15
182			min	-168.104	9	-.688	4	-.007	3	0	3	0	3	0	4
183		16	max	162.696	2	-.202	15	.014	1	0	2	0	2	0	15
184			min	-168.16	9	-.866	4	-.007	3	0	3	0	3	0	4
185		17	max	162.629	2	-.244	15	.014	1	0	2	0	2	0	15
186			min	-168.216	9	-1.044	4	-.007	3	0	3	0	3	0	4
187		18	max	162.562	2	-.285	15	.014	1	0	2	0	2	0	15
188			min	-168.272	9	-1.222	4	-.007	3	0	3	0	3	0	4
189		19	max	162.495	2	-.327	15	.014	1	0	2	0	2	0	1
190			min	-168.328	9	-1.4	4	-.007	3	0	3	0	3	0	1
191	M8	1	max	1706.612	1	0	1	.683	1	0	1	0	15	0	1
192			min	-391.186	3	0	1	-.219	3	0	1	0	1	0	1
193		2	max	1706.676	1	0	1	.683	1	0	1	0	1	0	1
194			min	-391.137	3	0	1	-.219	3	0	1	0	3	0	1
195		3	max	1706.741	1	0	1	.683	1	0	1	0	1	0	1
196			min	-391.089	3	0	1	-.219	3	0	1	0	3	0	1
197		4	max	1706.806	1	0	1	.683	1	0	1	0	1	0	1
198			min	-391.04	3	0	1	-.219	3	0	1	0	3	0	1
199		5	max	1706.87	1	0	1	.683	1	0	1	0	1	0	1
200			min	-390.992	3	0	1	-.219	3	0	1	0	3	0	1
201		6	max	1706.935	1	0	1	.683	1	0	1	0	1	0	1
202			min	-390.943	3	0	1	-.219	3	0	1	0	3	0	1
203		7	max	1707	1	0	1	.683	1	0	1	0	1	0	1
204			min	-390.895	3	0	1	-.219	3	0	1	0	3	0	1
205		8	max	1707.065	1	0	1	.683	1	0	1	0	1	0	1
206			min	-390.846	3	0	1	-.219	3	0	1	0	3	0	1
207		9	max	1707.129	1	0	1	.683	1	0	1	0	1	0	1
208			min	-390.798	3	0	1	-.219	3	0	1	0	3	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1707.194	1	0	1	.683	1	0	1	0	1	0	1
210			min	-390.749	3	0	1	-.219	3	0	1	0	3	0	1
211		11	max	1707.259	1	0	1	.683	1	0	1	0	1	0	1
212			min	-390.701	3	0	1	-.219	3	0	1	0	3	0	1
213		12	max	1707.323	1	0	1	.683	1	0	1	0	1	0	1
214			min	-390.652	3	0	1	-.219	3	0	1	0	3	0	1
215		13	max	1707.388	1	0	1	.683	1	0	1	0	1	0	1
216			min	-390.603	3	0	1	-.219	3	0	1	0	3	0	1
217		14	max	1707.453	1	0	1	.683	1	0	1	0	1	0	1
218			min	-390.555	3	0	1	-.219	3	0	1	0	3	0	1
219		15	max	1707.517	1	0	1	.683	1	0	1	0	1	0	1
220			min	-390.506	3	0	1	-.219	3	0	1	0	3	0	1
221		16	max	1707.582	1	0	1	.683	1	0	1	0	1	0	1
222			min	-390.458	3	0	1	-.219	3	0	1	0	3	0	1
223		17	max	1707.647	1	0	1	.683	1	0	1	0	1	0	1
224			min	-390.409	3	0	1	-.219	3	0	1	0	3	0	1
225		18	max	1707.712	1	0	1	.683	1	0	1	.001	1	0	1
226			min	-390.361	3	0	1	-.219	3	0	1	0	3	0	1
227		19	max	1707.776	1	0	1	.683	1	0	1	.001	1	0	1
228			min	-390.312	3	0	1	-.219	3	0	1	0	3	0	1
229	M10	1	max	426.375	1	.648	4	-.004	15	.001	1	0	2	0	1
230			min	-354.828	3	.155	15	-.108	1	0	3	0	3	0	1
231		2	max	426.472	1	.61	4	-.004	15	.001	1	0	2	0	15
232			min	-354.755	3	.146	15	-.108	1	0	3	0	3	0	4
233		3	max	426.568	1	.572	4	-.004	15	.001	1	0	2	0	15
234			min	-354.683	3	.137	15	-.108	1	0	3	0	3	0	4
235		4	max	426.664	1	.535	4	-.004	15	.001	1	0	10	0	15
236			min	-354.611	3	.129	15	-.108	1	0	3	0	3	0	4
237		5	max	426.761	1	.497	4	-.004	15	.001	1	0	10	0	15
238			min	-354.539	3	.12	15	-.108	1	0	3	0	3	0	4
239		6	max	426.857	1	.459	4	-.004	15	.001	1	0	15	0	15
240			min	-354.466	3	.111	15	-.108	1	0	3	0	3	0	4
241		7	max	426.953	1	.421	4	-.004	15	.001	1	0	15	0	15
242			min	-354.394	3	.102	15	-.108	1	0	3	0	1	0	4
243		8	max	427.05	1	.383	4	-.004	15	.001	1	0	15	0	15
244			min	-354.322	3	.093	15	-.108	1	0	3	0	1	0	4
245		9	max	427.146	1	.345	4	-.004	15	.001	1	0	15	0	15
246			min	-354.249	3	.084	15	-.108	1	0	3	0	1	0	4
247		10	max	427.242	1	.308	4	-.004	15	.001	1	0	15	0	15
248			min	-354.177	3	.075	15	-.108	1	0	3	0	1	0	4
249		11	max	427.339	1	.27	4	-.004	15	.001	1	0	15	0	15
250			min	-354.105	3	.066	15	-.108	1	0	3	0	1	0	4
251		12	max	427.435	1	.232	4	-.004	15	.001	1	0	15	0	15
252			min	-354.033	3	.057	15	-.108	1	0	3	0	1	0	4
253		13	max	427.532	1	.194	4	-.004	15	.001	1	0	15	0	15
254			min	-353.96	3	.049	15	-.108	1	0	3	0	1	0	4
255		14	max	427.628	1	.156	4	-.004	15	.001	1	0	15	0	15
256			min	-353.888	3	.028	1	-.108	1	0	3	0	1	0	4
257		15	max	427.724	1	.119	3	-.004	15	.001	1	0	15	0	15
258			min	-353.816	3	-.002	1	-.108	1	0	3	0	1	0	4
259		16	max	427.821	1	.097	3	-.004	15	.001	1	0	15	0	15
260			min	-353.744	3	-.031	1	-.108	1	0	3	0	1	0	4
261		17	max	427.917	1	.075	3	-.004	15	.001	1	0	15	0	15
262			min	-353.671	3	-.061	1	-.108	1	0	3	0	1	0	4
263		18	max	428.013	1	.053	3	-.004	15	.001	1	0	15	0	15
264			min	-353.599	3	-.09	1	-.108	1	0	3	0	1	0	4
265		19	max	428.11	1	.031	3	-.004	15	.001	1	0	15	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-353.527	3	- .12	1	- .108	1	0	3	0	1	0	4
267			max	34.161	10	1.816	4	.818	1	.001	1	0	3	0	4
268			min	-118.68	1	.428	15	.018	12	0	15	-.002	1	0	15
269		2	max	34.105	10	1.638	4	.818	1	.001	1	0	3	0	4
270			min	-118.747	1	.386	15	.018	12	0	15	-.002	1	0	15
271		3	max	34.049	10	1.46	4	.818	1	.001	1	0	3	0	4
272			min	-118.814	1	.344	15	.018	12	0	15	-.002	1	0	3
273		4	max	33.994	10	1.282	4	.818	1	.001	1	0	3	0	15
274			min	-118.882	1	.302	15	.018	12	0	15	-.001	1	0	4
275		5	max	33.938	10	1.104	4	.818	1	.001	1	0	3	0	15
276		min	-118.949	1	.26	15	.018	12	0	15	-.001	1	0	4	
277	6	max	33.882	10	.926	4	.818	1	.001	1	0	3	0	15	
278		min	-119.016	1	.218	15	.018	12	0	15	-.001	1	0	4	
279	7	max	33.826	10	.748	4	.818	1	.001	1	0	3	0	15	
280		min	-119.083	1	.176	15	.018	12	0	15	0	1	0	4	
281	8	max	33.77	10	.57	4	.818	1	.001	1	0	3	0	15	
282		min	-119.15	1	.135	15	.018	12	0	15	0	1	0	4	
283	9	max	33.714	10	.392	4	.818	1	.001	1	0	3	0	15	
284		min	-119.217	1	.093	15	.018	12	0	15	0	1	-.001	4	
285	10	max	33.658	10	.214	4	.818	1	.001	1	0	3	0	15	
286		min	-119.284	1	.051	15	.018	12	0	15	0	1	-.001	4	
287	11	max	33.602	10	.036	4	.818	1	.001	1	0	3	0	15	
288		min	-119.351	1	.002	3	.018	12	0	15	0	1	-.001	4	
289	12	max	33.546	10	-.033	15	.818	1	.001	1	0	3	0	15	
290		min	-119.418	1	-.142	4	.018	12	0	15	0	1	-.001	4	
291	13	max	33.49	10	-.075	15	.818	1	.001	1	0	3	0	15	
292		min	-119.485	1	-.32	4	.018	12	0	15	0	10	-.001	4	
293	14	max	33.434	10	-.116	15	.818	1	.001	1	0	1	0	15	
294		min	-119.552	1	-.498	4	.018	12	0	15	0	15	-.001	4	
295	15	max	33.379	10	-.158	15	.818	1	.001	1	0	1	0	15	
296		min	-119.62	1	-.676	4	.018	12	0	15	0	15	0	4	
297	16	max	33.323	10	-.2	15	.818	1	.001	1	0	1	0	15	
298		min	-119.687	1	-.854	4	.018	12	0	15	0	15	0	4	
299	17	max	33.267	10	-.242	15	.818	1	.001	1	0	1	0	15	
300		min	-119.754	1	-1.032	4	.018	12	0	15	0	15	0	4	
301	18	max	33.211	10	-.284	15	.818	1	.001	1	0	1	0	15	
302		min	-119.821	1	-1.21	4	.018	12	0	15	0	15	0	4	
303	19	max	33.155	10	-.326	15	.818	1	.001	1	.001	1	0	1	
304		min	-119.888	1	-1.388	4	.018	12	0	15	0	15	0	1	
305	M12	1	max	571.012	1	0	1	3.264	1	0	1	0	1	0	1
306			min	-111.847	3	0	1	.099	15	0	1	0	3	0	1
307		2	max	571.076	1	0	1	3.264	1	0	1	0	1	0	1
308			min	-111.799	3	0	1	.099	15	0	1	0	15	0	1
309		3	max	571.141	1	0	1	3.264	1	0	1	0	1	0	1
310			min	-111.75	3	0	1	.099	15	0	1	0	15	0	1
311		4	max	571.206	1	0	1	3.264	1	0	1	0	1	0	1
312			min	-111.702	3	0	1	.099	15	0	1	0	15	0	1
313		5	max	571.27	1	0	1	3.264	1	0	1	.001	1	0	1
314			min	-111.653	3	0	1	.099	15	0	1	0	15	0	1
315	6	max	571.335	1	0	1	3.264	1	0	1	.001	1	0	1	
316		min	-111.605	3	0	1	.099	15	0	1	0	15	0	1	
317	7	max	571.4	1	0	1	3.264	1	0	1	.002	1	0	1	
318		min	-111.556	3	0	1	.099	15	0	1	0	15	0	1	
319	8	max	571.464	1	0	1	3.264	1	0	1	.002	1	0	1	
320		min	-111.508	3	0	1	.099	15	0	1	0	15	0	1	
321	9	max	571.529	1	0	1	3.264	1	0	1	.002	1	0	1	
322		min	-111.459	3	0	1	.099	15	0	1	0	15	0	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	571.594	1	0	1	3.264	1	0	1	.003	1	0	1
324		min	-111.411	3	0	1	.099	15	0	1	0	15	0	1
325	11	max	571.659	1	0	1	3.264	1	0	1	.003	1	0	1
326		min	-111.362	3	0	1	.099	15	0	1	0	15	0	1
327	12	max	571.723	1	0	1	3.264	1	0	1	.003	1	0	1
328		min	-111.314	3	0	1	.099	15	0	1	0	15	0	1
329	13	max	571.788	1	0	1	3.264	1	0	1	.004	1	0	1
330		min	-111.265	3	0	1	.099	15	0	1	0	15	0	1
331	14	max	571.853	1	0	1	3.264	1	0	1	.004	1	0	1
332		min	-111.217	3	0	1	.099	15	0	1	0	15	0	1
333	15	max	571.917	1	0	1	3.264	1	0	1	.004	1	0	1
334		min	-111.168	3	0	1	.099	15	0	1	0	15	0	1
335	16	max	571.982	1	0	1	3.264	1	0	1	.004	1	0	1
336		min	-111.119	3	0	1	.099	15	0	1	0	15	0	1
337	17	max	572.047	1	0	1	3.264	1	0	1	.005	1	0	1
338		min	-111.071	3	0	1	.099	15	0	1	0	15	0	1
339	18	max	572.112	1	0	1	3.264	1	0	1	.005	1	0	1
340		min	-111.022	3	0	1	.099	15	0	1	0	15	0	1
341	19	max	572.176	1	0	1	3.264	1	0	1	.005	1	0	1
342		min	-110.974	3	0	1	.099	15	0	1	0	15	0	1
343	M1	1	max	111.122	1	341.698	3	-1.981	15	0	.126	1	.015	1
344		min	3.409	15	-416.413	1	-63.948	1	0	3	.004	15	-.01	3
345	2	max	111.194	1	341.496	3	-1.981	15	0	1	.112	1	.105	1
346		min	3.431	15	-416.683	1	-63.948	1	0	3	.003	15	-.084	3
347	3	max	126.281	1	6.829	9	-1.957	15	0	12	.097	1	.194	1
348		min	-6.675	3	-23.209	3	-63.543	1	0	1	.003	15	-.157	3
349	4	max	126.353	1	6.604	9	-1.957	15	0	12	.083	1	.194	1
350		min	-6.621	3	-23.411	3	-63.543	1	0	1	.003	15	-.152	3
351	5	max	126.425	1	6.379	9	-1.957	15	0	12	.07	1	.194	1
352		min	-6.567	3	-23.614	3	-63.543	1	0	1	.002	15	-.147	3
353	6	max	126.497	1	6.155	9	-1.957	15	0	12	.056	1	.194	1
354		min	-6.512	3	-23.816	3	-63.543	1	0	1	.002	15	-.142	3
355	7	max	126.57	1	5.93	9	-1.957	15	0	12	.042	1	.194	1
356		min	-6.458	3	-24.018	3	-63.543	1	0	1	.001	15	-.136	3
357	8	max	126.642	1	5.705	9	-1.957	15	0	12	.028	1	.194	1
358		min	-6.404	3	-24.22	3	-63.543	1	0	1	0	15	-.131	3
359	9	max	126.714	1	5.48	9	-1.957	15	0	12	.015	1	.195	1
360		min	-6.35	3	-24.423	3	-63.543	1	0	1	0	15	-.126	3
361	10	max	126.786	1	5.255	9	-1.957	15	0	12	0	1	.195	1
362		min	-6.296	3	-24.625	3	-63.543	1	0	1	0	15	-.121	3
363	11	max	126.859	1	5.031	9	-1.957	15	0	12	0	12	.195	1
364		min	-6.241	3	-24.827	3	-63.543	1	0	1	-.013	1	-.115	3
365	12	max	126.931	1	4.806	9	-1.957	15	0	12	0	12	.196	1
366		min	-6.187	3	-25.03	3	-63.543	1	0	1	-.027	1	-.11	3
367	13	max	127.003	1	4.581	9	-1.957	15	0	12	-.001	12	.196	1
368		min	-6.133	3	-25.232	3	-63.543	1	0	1	-.041	1	-.104	3
369	14	max	127.076	1	4.356	9	-1.957	15	0	12	-.002	15	.197	1
370		min	-6.079	3	-25.434	3	-63.543	1	0	1	-.054	1	-.099	3
371	15	max	127.148	1	4.132	9	-1.957	15	0	12	-.002	15	.197	1
372		min	-6.025	3	-25.636	3	-63.543	1	0	1	-.068	1	-.093	3
373	16	max	68.478	2	9.191	10	-1.978	15	0	1	-.003	15	.199	1
374		min	-34.329	3	-82.339	1	-64.158	1	0	12	-.083	1	-.088	3
375	17	max	68.55	2	8.966	10	-1.978	15	0	1	-.003	15	.217	1
376		min	-34.275	3	-82.609	1	-64.158	1	0	12	-.097	1	-.076	3
377	18	max	-3.416	15	467.941	1	-2.025	15	0	3	-.003	15	.117	1
378		min	-110.772	1	-160.21	3	-65.638	1	0	1	-.111	1	-.042	3
379	19	max	-3.394	15	467.672	1	-2.025	15	0	3	-.004	15	.016	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-110.7	1	-160.412	3	-65.638	1	0	1	-.125	1	-.007	3
381	M5	max	244.633	1	1128.874	3	-.06	10	0	1	.004	1	.021	3
382		min	6.558	12	-1376.046	1	-23.576	3	0	3	0	10	-.03	1
383		max	244.706	1	1128.672	3	-.06	10	0	1	0	2	.269	1
384		min	6.594	12	-1376.316	1	-23.576	3	0	3	-.002	3	-.224	3
385		max	292.65	1	10.164	9	2.697	3	0	3	0	2	.562	1
386		min	-32.018	3	-76.233	3	-.348	2	0	1	-.007	3	-.464	3
387		max	292.723	1	9.939	9	2.697	3	0	3	0	2	.565	1
388		min	-31.964	3	-76.435	3	-.348	2	0	1	-.007	3	-.447	3
389		max	292.795	1	9.715	9	2.697	3	0	3	0	2	.568	1
390		min	-31.909	3	-76.638	3	-.348	2	0	1	-.006	3	-.431	3
391		max	292.867	1	9.49	9	2.697	3	0	3	0	2	.572	1
392		min	-31.855	3	-76.84	3	-.348	2	0	1	-.006	3	-.414	3
393		max	292.939	1	9.265	9	2.697	3	0	3	0	2	.575	1
394		min	-31.801	3	-77.042	3	-.348	2	0	1	-.005	3	-.398	3
395		max	293.012	1	9.04	9	2.697	3	0	3	0	2	.578	1
396		min	-31.747	3	-77.245	3	-.348	2	0	1	-.004	3	-.381	3
397		max	293.084	1	8.816	9	2.697	3	0	3	0	2	.582	1
398		min	-31.693	3	-77.447	3	-.348	2	0	1	-.004	3	-.364	3
399		max	293.156	1	8.591	9	2.697	3	0	3	0	10	.586	1
400		min	-31.638	3	-77.649	3	-.348	2	0	1	-.003	3	-.347	3
401		max	293.229	1	8.366	9	2.697	3	0	3	0	10	.589	1
402		min	-31.584	3	-77.851	3	-.348	2	0	1	-.003	3	-.33	3
403		max	293.301	1	8.141	9	2.697	3	0	3	0	10	.593	1
404		min	-31.53	3	-78.054	3	-.348	2	0	1	-.002	1	-.313	3
405		max	293.373	1	7.916	9	2.697	3	0	3	0	10	.597	1
406		min	-31.476	3	-78.256	3	-.348	2	0	1	-.002	1	-.296	3
407		max	293.445	1	7.692	9	2.697	3	0	3	0	10	.6	1
408		min	-31.422	3	-78.458	3	-.348	2	0	1	-.002	1	-.279	3
409		max	293.518	1	7.467	9	2.697	3	0	3	0	15	.604	1
410		min	-31.367	3	-78.661	3	-.348	2	0	1	-.001	1	-.262	3
411		max	252.528	2	51.607	2	2.675	3	0	1	0	3	.609	1
412		min	-111.537	3	-148.354	3	-.347	2	0	15	-.001	1	-.245	3
413		max	252.6	2	51.338	2	2.675	3	0	1	0	3	.631	1
414		min	-111.483	3	-148.556	3	-.347	2	0	15	0	1	-.213	3
415		max	-7.016	12	1541.307	1	2.454	3	0	3	.001	3	.303	1
416		min	-245.279	1	-527.4	3	-.082	2	0	1	0	2	-.1	3
417		max	-6.98	12	1541.037	1	2.454	3	0	3	.002	3	.015	3
418		min	-245.207	1	-527.602	3	-.082	2	0	1	0	2	-.032	1
419	M9	max	110.618	1	341.687	3	83.116	1	0	3	-.004	15	.015	1
420		min	3.392	15	-416.398	1	2.672	15	0	1	-.126	1	-.01	3
421		max	110.69	1	341.485	3	83.116	1	0	3	-.002	12	.105	1
422		min	3.414	15	-416.668	1	2.672	15	0	1	-.107	1	-.084	3
423		max	126.356	1	6.808	9	60.033	1	0	1	.002	3	.194	1
424		min	-6.276	3	-23.154	3	.787	12	0	15	-.088	1	-.157	3
425		max	126.428	1	6.583	9	60.033	1	0	1	.003	3	.194	1
426		min	-6.222	3	-23.357	3	.787	12	0	15	-.075	1	-.152	3
427		max	126.5	1	6.359	9	60.033	1	0	1	.003	3	.194	1
428		min	-6.168	3	-23.559	3	.787	12	0	15	-.062	1	-.147	3
429		max	126.572	1	6.134	9	60.033	1	0	1	.003	3	.194	1
430		min	-6.114	3	-23.761	3	.787	12	0	15	-.049	1	-.142	3
431		max	126.645	1	5.909	9	60.033	1	0	1	.003	3	.194	1
432		min	-6.06	3	-23.963	3	.787	12	0	15	-.036	1	-.136	3
433		max	126.717	1	5.684	9	60.033	1	0	1	.003	3	.194	1
434		min	-6.005	3	-24.166	3	.787	12	0	15	-.023	1	-.131	3
435		max	126.789	1	5.459	9	60.033	1	0	1	.004	3	.194	1
436		min	-5.951	3	-24.368	3	.787	12	0	15	-.01	1	-.126	3







Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494		min	1.981	15	-415.856	1	3.409	15	-.015	1	.004	15	0	3
495	M16	max	-.989	12	468.245	1	-3.387	15	.007	3	.123	1	0	1
496		min	-63.787	1	-160.426	3	-110.475	1	-.016	1	.004	15	0	3
497		max	-.989	12	330.371	1	-2.594	15	.007	3	.037	1	.122	3
498		min	-63.787	1	-113.285	3	-84.565	1	-.016	1	.001	15	-.355	1
499		max	-.989	12	192.498	1	-1.801	15	.007	3	0	12	.201	3
500		min	-63.787	1	-66.143	3	-58.656	1	-.016	1	-.027	1	-.587	1
501		max	-.989	12	54.624	1	-1.009	15	.007	3	-.002	15	.239	3
502		min	-63.787	1	-19.002	3	-32.746	1	-.016	1	-.068	1	-.697	1
503		max	-.989	12	28.139	3	-.216	15	.007	3	-.003	15	.235	3
504		min	-63.787	1	-83.25	1	-6.836	1	-.016	1	-.085	1	-.684	1
505		max	-.989	12	75.28	3	19.073	1	.007	3	-.002	15	.189	3
506		min	-63.787	1	-221.124	1	.34	12	-.016	1	-.08	1	-.549	1
507		max	-.989	12	122.421	3	44.983	1	.007	3	-.002	15	.101	3
508		min	-63.787	1	-358.998	1	1.114	12	-.016	1	-.051	1	-.291	1
509		max	-.989	12	169.562	3	70.892	1	.007	3	.001	2	.089	1
510		min	-63.787	1	-496.872	1	1.888	12	-.016	1	-.002	3	-.028	3
511		max	-.989	12	216.703	3	96.802	1	.007	3	.075	1	.592	1
512		min	-63.787	1	-634.746	1	2.661	12	-.016	1	.001	12	-.2	3
513		max	-2.025	15	-16.582	15	122.712	1	0	15	.172	1	1.218	1
514		min	-65.474	1	-772.62	1	-5.3	3	-.016	1	.005	12	-.414	3
515		max	-2.025	15	634.746	1	-2.771	12	.016	1	.075	1	.592	1
516		min	-65.474	1	-216.703	3	-96.568	1	-.007	3	.002	12	-.2	3
517		max	-2.025	15	496.872	1	-1.998	12	.016	1	0	2	.089	1
518		min	-65.474	1	-169.562	3	-70.658	1	-.007	3	0	3	-.028	3
519		max	-2.025	15	358.998	1	-1.224	12	.016	1	-.002	12	.101	3
520		min	-65.474	1	-122.421	3	-44.748	1	-.007	3	-.051	1	-.291	1
521		max	-2.025	15	221.124	1	-.45	12	.016	1	-.002	12	.189	3
522		min	-65.474	1	-75.28	3	-18.839	1	-.007	3	-.079	1	-.549	1
523		max	-2.025	15	83.25	1	7.071	1	.016	1	-.002	12	.235	3
524		min	-65.474	1	-28.139	3	.223	15	-.007	3	-.084	1	-.684	1
525		max	-2.025	15	19.002	3	32.981	1	.016	1	-.002	12	.239	3
526		min	-65.474	1	-54.624	1	1.016	15	-.007	3	-.066	1	-.697	1
527		max	-2.025	15	66.143	3	58.89	1	.016	1	0	12	.201	3
528		min	-65.474	1	-192.498	1	1.809	15	-.007	3	-.026	1	-.587	1
529		max	-2.025	15	113.285	3	84.8	1	.016	1	.038	1	.122	3
530		min	-65.474	1	-330.372	1	2.601	15	-.007	3	.001	15	-.355	1
531		max	-2.025	15	160.426	3	110.71	1	.016	1	.125	1	0	1
532		min	-65.474	1	-468.246	1	3.394	15	-.007	3	.004	15	0	3
533	M15	max	0	2	2.784	4	.024	3	0	1	0	1	0	1
534		min	-27.258	3	0	2	-.033	1	0	3	0	3	0	1
535		max	0	2	2.474	4	.024	3	0	1	0	1	0	2
536		min	-27.312	3	0	2	-.033	1	0	3	0	3	-.001	4
537		max	0	2	2.165	4	.024	3	0	1	0	1	0	2
538		min	-27.366	3	0	2	-.033	1	0	3	0	3	-.002	4
539		max	0	2	1.856	4	.024	3	0	1	0	1	0	2
540		min	-27.42	3	0	2	-.033	1	0	3	0	3	-.003	4
541		max	0	2	1.546	4	.024	3	0	1	0	1	0	2
542		min	-27.474	3	0	2	-.033	1	0	3	0	3	-.004	4
543		max	0	2	1.237	4	.024	3	0	1	0	1	0	2
544		min	-27.528	3	0	2	-.033	1	0	3	0	3	-.005	4
545		max	0	2	.928	4	.024	3	0	1	0	3	0	2
546		min	-27.582	3	0	2	-.033	1	0	3	0	1	-.005	4
547		max	0	2	.619	4	.024	3	0	1	0	3	0	2
548		min	-27.636	3	0	2	-.033	1	0	3	0	1	-.006	4
549		max	0	2	.309	4	.024	3	0	1	0	3	0	2
550		min	-27.69	3	0	2	-.033	1	0	3	0	1	-.006	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	2	0	1	.024	3	0	1	0	3	0	2
552			min	-27.744	3	0	1	-.033	1	0	3	0	1	-.006	4
553		11	max	0	2	0	2	.024	3	0	1	0	3	0	2
554			min	-27.798	3	-.309	4	-.033	1	0	3	0	1	-.006	4
555		12	max	0	2	0	2	.024	3	0	1	0	3	0	2
556			min	-27.852	3	-.619	4	-.033	1	0	3	0	1	-.006	4
557		13	max	0	2	0	2	.024	3	0	1	0	3	0	2
558			min	-27.906	3	-.928	4	-.033	1	0	3	0	1	-.005	4
559		14	max	0	2	0	2	.024	3	0	1	0	3	0	2
560			min	-27.96	3	-1.237	4	-.033	1	0	3	0	1	-.005	4
561		15	max	0	2	0	2	.024	3	0	1	0	3	0	2
562			min	-28.014	3	-1.546	4	-.033	1	0	3	0	1	-.004	4
563		16	max	0	2	0	2	.024	3	0	1	0	3	0	2
564			min	-28.068	3	-1.856	4	-.033	1	0	3	0	1	-.003	4
565		17	max	0	2	0	2	.024	3	0	1	0	3	0	2
566			min	-28.122	3	-2.165	4	-.033	1	0	3	0	1	-.002	4
567		18	max	0	2	0	2	.024	3	0	1	0	3	0	2
568			min	-28.176	3	-2.474	4	-.033	1	0	3	0	1	-.001	4
569		19	max	0	2	0	2	.024	3	0	1	0	3	0	1
570			min	-28.23	3	-2.784	4	-.033	1	0	3	0	1	0	1
571	M16A	1	max	-.722	10	2.784	4	.02	1	0	3	0	3	0	1
572			min	-27.756	3	.654	15	-.01	3	0	1	0	1	0	1
573		2	max	-.663	10	2.474	4	.02	1	0	3	0	3	0	15
574			min	-27.702	3	.582	15	-.01	3	0	1	0	1	-.001	4
575		3	max	-.603	10	2.165	4	.02	1	0	3	0	3	0	15
576			min	-27.648	3	.509	15	-.01	3	0	1	0	1	-.002	4
577		4	max	-.543	10	1.856	4	.02	1	0	3	0	3	0	15
578			min	-27.594	3	.436	15	-.01	3	0	1	0	1	-.003	4
579		5	max	-.483	10	1.546	4	.02	1	0	3	0	3	0	15
580			min	-27.54	3	.364	15	-.01	3	0	1	0	1	-.004	4
581		6	max	-.423	10	1.237	4	.02	1	0	3	0	3	-.001	15
582			min	-27.486	3	.291	15	-.01	3	0	1	0	1	-.005	4
583		7	max	-.363	10	.928	4	.02	1	0	3	0	3	-.001	15
584			min	-27.432	3	.218	15	-.01	3	0	1	0	1	-.005	4
585		8	max	-.303	10	.619	4	.02	1	0	3	0	3	-.001	15
586			min	-27.378	3	.145	15	-.01	3	0	1	0	1	-.006	4
587		9	max	-.243	10	.309	4	.02	1	0	3	0	3	-.001	15
588			min	-27.324	3	.073	15	-.01	3	0	1	0	1	-.006	4
589		10	max	-.183	10	0	1	.02	1	0	3	0	3	-.001	15
590			min	-27.27	3	0	1	-.01	3	0	1	0	1	-.006	4
591		11	max	-.123	10	-.073	15	.02	1	0	3	0	3	-.001	15
592			min	-27.216	3	-.309	4	-.01	3	0	1	0	1	-.006	4
593		12	max	-.063	10	-.145	15	.02	1	0	3	0	3	-.001	15
594			min	-27.162	3	-.619	4	-.01	3	0	1	0	1	-.006	4
595		13	max	-.003	10	-.218	15	.02	1	0	3	0	1	-.001	15
596			min	-27.108	3	-.928	4	-.01	3	0	1	0	4	-.005	4
597		14	max	.057	10	-.291	15	.02	1	0	3	0	1	-.001	15
598			min	-27.054	3	-1.237	4	-.01	3	0	1	0	3	-.005	4
599		15	max	.117	10	-.364	15	.02	1	0	3	0	1	0	15
600			min	-.27	3	-1.546	4	-.01	3	0	1	0	3	-.004	4
601		16	max	.177	10	-.436	15	.02	1	0	3	0	1	0	15
602			min	-26.946	3	-1.856	4	-.01	3	0	1	0	3	-.003	4
603		17	max	.237	10	-.509	15	.02	1	0	3	0	1	0	15
604			min	-26.892	3	-2.165	4	-.01	3	0	1	0	3	-.002	4
605		18	max	.297	10	-.582	15	.02	1	0	3	0	1	0	15
606			min	-26.838	3	-2.474	4	-.01	3	0	1	0	3	-.001	4
607		19	max	.357	10	-.654	15	.02	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-26.784	3	-2.784	4	-.01	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.012	1	-2.82e-5	15	NC	3	NC	3	
2			min	-.003	3	-.005	3	0	3	-9.135e-4	1	4867.067	2	2486.431	1	
3			2	max	.003	1	.006	2	.011	1	-2.709e-5	15	NC	3	NC	3
4				min	-.003	3	-.005	3	0	3	-8.777e-4	1	5268.035	2	2695.562	1
5			3	max	.003	1	.005	2	.01	1	-2.597e-5	15	NC	3	NC	3
6				min	-.002	3	-.004	3	0	3	-8.419e-4	1	5737.65	2	2941.623	1
7			4	max	.003	1	.005	2	.009	1	-2.485e-5	15	NC	3	NC	3
8				min	-.002	3	-.004	3	0	3	-8.061e-4	1	6291.34	2	3233.617	1
9			5	max	.002	1	.004	2	.008	1	-2.374e-5	15	NC	3	NC	3
10				min	-.002	3	-.004	3	0	3	-7.703e-4	1	6949.351	2	3583.563	1
11		6	max	.002	1	.004	2	.008	1	-2.262e-5	15	NC	1	NC	2	
12			min	-.002	3	-.004	3	0	3	-7.345e-4	1	7738.689	2	4007.795	1	
13		7	max	.002	1	.003	2	.007	1	-2.15e-5	15	NC	1	NC	2	
14			min	-.002	3	-.004	3	0	3	-6.987e-4	1	8696.047	2	4528.984	1	
15		8	max	.002	1	.003	2	.006	1	-2.039e-5	15	NC	1	NC	2	
16			min	-.002	3	-.003	3	0	3	-6.628e-4	1	9872.378	2	5179.356	1	
17		9	max	.002	1	.003	2	.005	1	-1.927e-5	15	NC	1	NC	2	
18			min	-.002	3	-.003	3	0	3	-6.27e-4	1	NC	1	6006.024	1	
19		10	max	.002	1	.002	2	.004	1	-1.815e-5	15	NC	1	NC	2	
20			min	-.001	3	-.003	3	0	3	-5.912e-4	1	NC	1	7080.181	1	
21		11	max	.001	1	.002	2	.004	1	-1.703e-5	15	NC	1	NC	2	
22			min	-.001	3	-.003	3	0	3	-5.554e-4	1	NC	1	8513.757	1	
23		12	max	.001	1	.002	2	.003	1	-1.592e-5	15	NC	1	NC	1	
24			min	-.001	3	-.002	3	0	3	-5.196e-4	1	NC	1	NC	1	
25		13	max	.001	1	.001	2	.002	1	-1.48e-5	15	NC	1	NC	1	
26			min	0	3	-.002	3	0	3	-4.838e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-1.368e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-4.48e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	-1.257e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-4.122e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.145e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-3.764e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.033e-5	15	NC	1	NC	1	
34			min	0	3	0	3	0	3	-3.406e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-9.217e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.048e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-6.598e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.689e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.223e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.104e-6	12	NC	1	NC	1	
41			2	max	0	1	0	2	0	12	1.554e-4	1	NC	1	NC	1
42				min	0	10	0	3	0	1	4.714e-6	15	NC	1	NC	1
43			3	max	0	1	0	2	0	12	1.886e-4	1	NC	1	NC	1
44				min	0	10	-.001	3	0	1	5.746e-6	15	NC	1	NC	1
45			4	max	0	1	0	2	0	12	2.218e-4	1	NC	1	NC	1
46				min	0	10	-.002	3	-.001	1	6.778e-6	15	NC	1	NC	1
47			5	max	0	1	0	2	0	3	2.55e-4	1	NC	1	NC	1
48				min	0	10	-.003	3	-.001	1	7.809e-6	15	NC	1	NC	1
49			6	max	0	1	0	2	0	3	2.882e-4	1	NC	1	NC	1
50				min	0	10	-.003	3	-.001	1	8.841e-6	15	NC	1	NC	1
51		7	max	0	1	0	2	0	3	3.214e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	10	-.004	3	0	1	9.872e-6	15	NC	1	NC	1
53		8 max	0	1	.001	2	0	3	3.546e-4	1	NC	1	NC	1
54		min	0	10	-.005	3	0	1	1.09e-5	15	NC	1	NC	1
55		9 max	0	1	.002	2	0	3	3.878e-4	1	NC	1	NC	1
56		min	0	10	-.005	3	0	1	1.194e-5	15	NC	1	NC	1
57		10 max	0	1	.002	2	0	1	4.21e-4	1	NC	1	NC	1
58		min	0	10	-.006	3	0	15	1.297e-5	15	NC	1	NC	1
59		11 max	0	1	.003	2	.001	1	4.542e-4	1	NC	1	NC	1
60		min	0	10	-.006	3	0	15	1.4e-5	15	NC	1	NC	1
61		12 max	0	1	.003	2	.002	1	4.874e-4	1	NC	1	NC	1
62		min	0	10	-.006	3	0	15	1.503e-5	15	NC	1	NC	1
63		13 max	0	1	.004	2	.002	1	5.206e-4	1	NC	1	NC	1
64		min	0	10	-.007	3	0	15	1.606e-5	15	NC	1	NC	1
65		14 max	0	1	.005	2	.003	1	5.538e-4	1	NC	1	NC	1
66		min	0	10	-.007	3	0	15	1.709e-5	15	9790.049	2	NC	1
67		15 max	.001	1	.006	2	.004	1	5.87e-4	1	NC	3	NC	1
68		min	0	10	-.007	3	0	15	1.813e-5	15	8254.389	2	NC	1
69		16 max	.001	1	.007	2	.004	1	6.201e-4	1	NC	3	NC	1
70		min	0	10	-.007	3	0	15	1.916e-5	15	7059.582	2	NC	1
71		17 max	.001	1	.008	2	.005	1	6.533e-4	1	NC	3	NC	2
72		min	0	10	-.007	3	0	15	2.019e-5	15	6120.811	2	9188.924	1
73		18 max	.001	1	.009	2	.006	1	6.865e-4	1	NC	3	NC	2
74		min	0	10	-.007	3	0	15	2.122e-5	15	5376.907	2	8183.234	1
75		19 max	.001	1	.01	1	.006	1	7.197e-4	1	NC	3	NC	2
76		min	0	10	-.007	3	0	15	2.225e-5	15	4763.986	1	7439.947	1
77	M4	1 max	.003	1	.007	2	0	15	-2.481e-5	15	NC	1	NC	2
78		min	0	3	-.005	3	-.005	1	-8.114e-4	1	NC	1	4183.906	1
79		2 max	.003	1	.007	2	0	15	-2.481e-5	15	NC	1	NC	2
80		min	0	3	-.005	3	-.004	1	-8.114e-4	1	NC	1	4564.301	1
81		3 max	.002	1	.006	2	0	15	-2.481e-5	15	NC	1	NC	2
82		min	0	3	-.005	3	-.004	1	-8.114e-4	1	NC	1	5017.029	1
83		4 max	.002	1	.006	2	0	15	-2.481e-5	15	NC	1	NC	2
84		min	0	3	-.004	3	-.003	1	-8.114e-4	1	NC	1	5561.171	1
85		5 max	.002	1	.006	2	0	15	-2.481e-5	15	NC	1	NC	2
86		min	0	3	-.004	3	-.003	1	-8.114e-4	1	NC	1	6222.72	1
87		6 max	.002	1	.005	2	0	15	-2.481e-5	15	NC	1	NC	2
88		min	0	3	-.004	3	-.003	1	-8.114e-4	1	NC	1	7037.826	1
89		7 max	.002	1	.005	2	0	15	-2.481e-5	15	NC	1	NC	2
90		min	0	3	-.003	3	-.002	1	-8.114e-4	1	NC	1	8057.988	1
91		8 max	.002	1	.004	2	0	15	-2.481e-5	15	NC	1	NC	2
92		min	0	3	-.003	3	-.002	1	-8.114e-4	1	NC	1	9358.62	1
93		9 max	.002	1	.004	2	0	15	-2.481e-5	15	NC	1	NC	1
94		min	0	3	-.003	3	-.002	1	-8.114e-4	1	NC	1	NC	1
95		10 max	.001	1	.004	2	0	15	-2.481e-5	15	NC	1	NC	1
96		min	0	3	-.003	3	-.001	1	-8.114e-4	1	NC	1	NC	1
97		11 max	.001	1	.003	2	0	15	-2.481e-5	15	NC	1	NC	1
98		min	0	3	-.002	3	-.001	1	-8.114e-4	1	NC	1	NC	1
99		12 max	.001	1	.003	2	0	15	-2.481e-5	15	NC	1	NC	1
100		min	0	3	-.002	3	0	1	-8.114e-4	1	NC	1	NC	1
101		13 max	0	1	.002	2	0	15	-2.481e-5	15	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-8.114e-4	1	NC	1	NC	1
103		14 max	0	1	.002	2	0	15	-2.481e-5	15	NC	1	NC	1
104		min	0	3	-.001	3	0	1	-8.114e-4	1	NC	1	NC	1
105		15 max	0	1	.002	2	0	15	-2.481e-5	15	NC	1	NC	1
106		min	0	3	-.001	3	0	1	-8.114e-4	1	NC	1	NC	1
107		16 max	0	1	.001	2	0	15	-2.481e-5	15	NC	1	NC	1
108		min	0	3	0	3	0	1	-8.114e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-2.481e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-8.114e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-2.481e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-8.114e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-2.481e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-8.114e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.02	2	.004	1	1.832e-4	1	NC	3	NC	2
116			min	-.009	3	-.013	3	-.002	3	3.139e-6	10	1537.557	2	7981.469	1
117		2	max	.01	1	.018	2	.003	1	1.707e-4	1	NC	3	NC	2
118			min	-.008	3	-.013	3	-.002	3	2.48e-6	10	1639.671	2	8648.546	1
119		3	max	.009	1	.017	2	.003	1	1.583e-4	1	NC	3	NC	2
120			min	-.008	3	-.012	3	-.002	3	1.822e-6	10	1755.99	2	9440.039	1
121		4	max	.008	1	.016	2	.003	1	1.458e-4	1	NC	3	NC	1
122			min	-.007	3	-.011	3	-.002	3	1.163e-6	10	1889.347	2	NC	1
123		5	max	.008	1	.015	2	.003	1	1.371e-4	3	NC	3	NC	1
124			min	-.007	3	-.011	3	-.001	3	5.041e-7	10	2043.384	2	NC	1
125		6	max	.007	1	.014	2	.002	1	1.335e-4	3	NC	3	NC	1
126			min	-.006	3	-.01	3	-.001	3	-1.547e-7	10	2222.869	2	NC	1
127		7	max	.007	1	.012	2	.002	1	1.3e-4	3	NC	3	NC	1
128			min	-.006	3	-.009	3	-.001	3	-8.134e-7	10	2434.164	2	NC	1
129		8	max	.006	1	.011	2	.002	1	1.264e-4	3	NC	3	NC	1
130			min	-.005	3	-.009	3	-.001	3	-2.064e-6	2	2685.941	2	NC	1
131		9	max	.006	1	.01	2	.002	1	1.228e-4	3	NC	3	NC	1
132			min	-.005	3	-.008	3	0	3	-5.805e-6	2	2990.35	2	NC	1
133		10	max	.005	1	.009	2	.001	1	1.192e-4	3	NC	3	NC	1
134			min	-.004	3	-.007	3	0	3	-9.546e-6	2	3364.935	2	NC	1
135		11	max	.005	1	.008	2	.001	1	1.157e-4	3	NC	3	NC	1
136			min	-.004	3	-.007	3	0	3	-1.329e-5	2	3836.016	2	NC	1
137		12	max	.004	1	.007	2	0	1	1.121e-4	3	NC	3	NC	1
138			min	-.003	3	-.006	3	0	3	-1.703e-5	2	4444.95	2	NC	1
139		13	max	.003	1	.006	2	0	1	1.085e-4	3	NC	3	NC	1
140			min	-.003	3	-.005	3	0	3	-2.077e-5	2	5260.662	2	NC	1
141		14	max	.003	1	.005	2	0	1	1.049e-4	3	NC	3	NC	1
142			min	-.002	3	-.004	3	0	3	-2.451e-5	2	6407.207	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.014e-4	3	NC	3	NC	1
144			min	-.002	3	-.003	3	0	3	-2.825e-5	2	8132.687	2	NC	1
145		16	max	.002	1	.003	2	0	1	9.779e-5	3	NC	1	NC	1
146			min	-.001	3	-.003	3	0	3	-3.199e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	9.421e-5	3	NC	1	NC	1
148			min	0	3	-.002	3	0	3	-3.573e-5	2	NC	1	NC	1
149		18	max	0	1	0	2	0	1	9.064e-5	3	NC	1	NC	1
150			min	0	3	0	3	0	3	-3.947e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	8.706e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.322e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.941e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-3.943e-5	3	NC	1	NC	1
155		2	max	0	9	.001	2	0	3	1.716e-5	1	NC	1	NC	1
156			min	0	2	-.001	3	0	2	-3.019e-5	3	NC	1	NC	1
157		3	max	0	9	.003	2	0	3	1.614e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-2.095e-5	3	NC	1	NC	1
159		4	max	0	9	.004	2	0	3	1.511e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	1	-1.171e-5	3	NC	1	NC	1
161		5	max	0	9	.005	2	0	3	1.409e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	1	-2.473e-6	3	9201.506	2	NC	1
163		6	max	0	9	.006	1	0	3	1.306e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	3.299e-7	15	7350.17	1	NC	1
165		7	max	0	9	.008	1	0	3	1.601e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	0	2	-.008	3	0	1	3.433e-7	15	6032.798	1	NC	1
167		8	max	0	9	.009	1	.001	3	2.525e-5	3	NC	3	NC	1
168			min	0	2	-.01	3	0	1	-2.291e-6	2	5073.038	1	NC	1
169		9	max	0	9	.011	1	.001	3	3.449e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	-5.391e-6	2	4341.194	1	NC	1
171		10	max	0	9	.012	1	.001	3	4.372e-5	3	NC	3	NC	1
172			min	0	2	-.012	3	-.001	1	-8.492e-6	2	3764.944	1	NC	1
173		11	max	.001	9	.014	1	.001	3	5.296e-5	3	NC	3	NC	1
174			min	-.001	2	-.013	3	-.001	1	-1.159e-5	2	3300.538	1	NC	1
175		12	max	.001	9	.016	1	.001	3	6.22e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-1.469e-5	2	2919.757	1	NC	1
177		13	max	.001	9	.018	1	.001	3	7.144e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.001	1	-1.779e-5	2	2603.434	1	NC	1
179		14	max	.001	9	.02	1	.001	3	8.068e-5	3	NC	3	NC	1
180			min	-.001	2	-.016	3	-.001	1	-2.089e-5	2	2337.996	1	NC	1
181		15	max	.001	9	.022	1	.001	3	8.992e-5	3	NC	3	NC	1
182			min	-.001	2	-.017	3	-.002	1	-2.399e-5	2	2113.496	1	NC	1
183		16	max	.002	9	.024	1	.001	3	9.916e-5	3	NC	3	NC	1
184			min	-.002	2	-.017	3	-.002	1	-2.709e-5	2	1922.443	1	NC	1
185		17	max	.002	9	.026	1	.001	3	1.084e-4	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-3.019e-5	2	1759.074	1	NC	1
187		18	max	.002	9	.028	1	.001	3	1.176e-4	3	NC	3	NC	1
188			min	-.002	2	-.019	3	-.002	1	-3.33e-5	2	1618.877	1	NC	1
189		19	max	.002	9	.031	1	.001	3	1.269e-4	3	NC	3	NC	1
190			min	-.002	2	-.02	3	-.002	1	-3.64e-5	2	1498.275	1	NC	1
191	M8	1	max	.008	1	.023	2	.002	1	-4.195e-7	10	NC	1	NC	2
192			min	-.002	3	-.015	3	0	3	-1.02e-4	3	NC	1	8967.953	1
193		2	max	.008	1	.022	2	.002	1	-4.195e-7	10	NC	1	NC	2
194			min	-.002	3	-.014	3	0	3	-1.02e-4	3	NC	1	9777.538	1
195		3	max	.007	1	.02	2	.002	1	-4.195e-7	10	NC	1	NC	1
196			min	-.002	3	-.013	3	0	3	-1.02e-4	3	NC	1	NC	1
197		4	max	.007	1	.019	2	.002	1	-4.195e-7	10	NC	1	NC	1
198			min	-.002	3	-.012	3	0	3	-1.02e-4	3	NC	1	NC	1
199		5	max	.006	1	.018	2	.001	1	-4.195e-7	10	NC	1	NC	1
200			min	-.001	3	-.011	3	0	3	-1.02e-4	3	NC	1	NC	1
201		6	max	.006	1	.016	2	.001	1	-4.195e-7	10	NC	1	NC	1
202			min	-.001	3	-.011	3	0	3	-1.02e-4	3	NC	1	NC	1
203		7	max	.005	1	.015	2	.001	1	-4.195e-7	10	NC	1	NC	1
204			min	-.001	3	-.01	3	0	3	-1.02e-4	3	NC	1	NC	1
205		8	max	.005	1	.014	2	0	1	-4.195e-7	10	NC	1	NC	1
206			min	-.001	3	-.009	3	0	3	-1.02e-4	3	NC	1	NC	1
207		9	max	.005	1	.013	2	0	1	-4.195e-7	10	NC	1	NC	1
208			min	-.001	3	-.008	3	0	3	-1.02e-4	3	NC	1	NC	1
209		10	max	.004	1	.011	2	0	1	-4.195e-7	10	NC	1	NC	1
210			min	0	3	-.007	3	0	3	-1.02e-4	3	NC	1	NC	1
211		11	max	.004	1	.01	2	0	1	-4.195e-7	10	NC	1	NC	1
212			min	0	3	-.007	3	0	3	-1.02e-4	3	NC	1	NC	1
213		12	max	.003	1	.009	2	0	1	-4.195e-7	10	NC	1	NC	1
214			min	0	3	-.006	3	0	3	-1.02e-4	3	NC	1	NC	1
215		13	max	.003	1	.008	2	0	1	-4.195e-7	10	NC	1	NC	1
216			min	0	3	-.005	3	0	3	-1.02e-4	3	NC	1	NC	1
217		14	max	.002	1	.006	2	0	1	-4.195e-7	10	NC	1	NC	1
218			min	0	3	-.004	3	0	3	-1.02e-4	3	NC	1	NC	1
219		15	max	.002	1	.005	2	0	1	-4.195e-7	10	NC	1	NC	1
220			min	0	3	-.003	3	0	3	-1.02e-4	3	NC	1	NC	1
221		16	max	.001	1	.004	2	0	1	-4.195e-7	10	NC	1	NC	1
222			min	0	3	-.002	3	0	3	-1.02e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-4.195e-7	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.02e-4	3	NC	1	NC	1
225		18	max	0	1	.001	2	0	1	-4.195e-7	10	NC	1	NC	1
226			min	0	3	0	3	0	3	-1.02e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-4.195e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.02e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.006	2	0	3	8.342e-4	1	NC	3	NC	1
230			min	-.003	3	-.005	3	-.002	1	-1.649e-4	3	4876.81	2	NC	1
231		2	max	.003	1	.006	2	0	3	7.913e-4	1	NC	3	NC	1
232			min	-.002	3	-.005	3	-.002	1	-1.606e-4	3	5278.756	2	NC	1
233		3	max	.003	1	.005	2	0	3	7.485e-4	1	NC	3	NC	1
234			min	-.002	3	-.004	3	-.001	1	-1.562e-4	3	5749.547	2	NC	1
235		4	max	.003	1	.005	2	0	3	7.057e-4	1	NC	3	NC	1
236			min	-.002	3	-.004	3	-.001	1	-1.519e-4	3	6304.659	2	NC	1
237		5	max	.002	1	.004	2	0	3	6.629e-4	1	NC	3	NC	1
238			min	-.002	3	-.004	3	-.001	1	-1.475e-4	3	6964.406	2	NC	1
239		6	max	.002	1	.004	2	0	3	6.201e-4	1	NC	1	NC	1
240			min	-.002	3	-.004	3	-.001	1	-1.432e-4	3	7755.885	2	NC	1
241		7	max	.002	1	.003	2	0	3	5.772e-4	1	NC	1	NC	1
242			min	-.002	3	-.004	3	0	1	-1.389e-4	3	8715.915	2	NC	1
243		8	max	.002	1	.003	2	0	3	5.344e-4	1	NC	1	NC	1
244			min	-.002	3	-.004	3	0	1	-1.345e-4	3	9895.631	2	NC	1
245		9	max	.002	1	.003	2	0	3	4.916e-4	1	NC	1	NC	1
246			min	-.001	3	-.003	3	0	1	-1.302e-4	3	NC	1	NC	1
247		10	max	.002	1	.002	2	0	3	4.488e-4	1	NC	1	NC	1
248			min	-.001	3	-.003	3	0	1	-1.258e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.06e-4	1	NC	1	NC	1
250			min	-.001	3	-.003	3	0	1	-1.215e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	3.631e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.172e-4	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	3.203e-4	1	NC	1	NC	1
254			min	0	3	-.002	3	0	1	-1.128e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	2.775e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.085e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.347e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.041e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.918e-4	1	NC	1	NC	1
260			min	0	3	-.001	3	0	1	-9.979e-5	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.49e-4	1	NC	1	NC	1
262			min	0	3	0	3	0	1	-9.545e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.062e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-9.11e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.339e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.676e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	3.955e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.016e-5	1	NC	1	NC	1
269		2	max	0	1	0	2	0	1	2.88e-5	3	NC	1	NC	1
270			min	0	10	0	3	0	3	-8.765e-5	1	NC	1	NC	1
271		3	max	0	1	0	2	0	10	1.805e-5	3	NC	1	NC	1
272			min	0	10	-.001	3	0	3	-1.451e-4	1	NC	1	NC	1
273		4	max	0	1	0	2	0	10	7.307e-6	3	NC	1	NC	1
274			min	0	10	-.002	3	0	3	-2.026e-4	1	NC	1	NC	1
275		5	max	0	1	0	2	0	10	-2.855e-6	12	NC	1	NC	1
276			min	0	10	-.003	3	0	1	-2.601e-4	1	NC	1	NC	1
277		6	max	0	1	0	2	0	15	-9.482e-6	12	NC	1	NC	1
278			min	0	10	-.004	3	-.001	1	-3.176e-4	1	NC	1	NC	1
279		7	max	0	1	0	2	0	15	-1.152e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	10	-.004	3	-.002	1	-3.751e-4	1	NC	1	NC	1
281		8	max	0	1	.001	2	0	15	-1.332e-5	15	NC	1	NC	1
282			min	0	10	-.005	3	-.003	1	-4.326e-4	1	NC	1	NC	1
283		9	max	0	1	.002	2	0	15	-1.513e-5	15	NC	1	NC	1
284			min	0	10	-.005	3	-.004	1	-4.9e-4	1	NC	1	NC	1
285		10	max	0	1	.002	2	0	15	-1.693e-5	15	NC	1	NC	2
286			min	0	10	-.006	3	-.005	1	-5.475e-4	1	NC	1	9957.519	1
287		11	max	0	1	.003	2	0	15	-1.874e-5	15	NC	1	NC	2
288			min	0	10	-.006	3	-.006	1	-6.05e-4	1	NC	1	8255.251	1
289		12	max	0	1	.003	2	0	15	-2.054e-5	15	NC	1	NC	2
290			min	0	10	-.006	3	-.007	1	-6.625e-4	1	NC	1	7025.216	1
291		13	max	0	1	.004	2	0	15	-2.234e-5	15	NC	1	NC	2
292			min	0	10	-.007	3	-.008	1	-7.2e-4	1	NC	1	6109.839	1
293		14	max	0	1	.005	2	0	15	-2.415e-5	15	NC	3	NC	2
294			min	0	10	-.007	3	-.009	1	-7.775e-4	1	9806.303	2	5413.337	1
295		15	max	.001	1	.006	1	0	15	-2.595e-5	15	NC	3	NC	2
296			min	0	10	-.007	3	-.009	1	-8.35e-4	1	8256.754	1	4874.825	1
297		16	max	.001	1	.007	1	0	15	-2.775e-5	15	NC	3	NC	2
298			min	0	10	-.007	3	-.01	1	-8.924e-4	1	7052.175	1	4454.222	1
299		17	max	.001	1	.008	1	0	15	-2.956e-5	15	NC	3	NC	2
300			min	0	10	-.007	3	-.011	1	-9.499e-4	1	6108.055	1	4124.456	1
301		18	max	.001	1	.009	1	0	15	-3.136e-5	15	NC	3	NC	2
302			min	0	10	-.007	3	-.012	1	-1.007e-3	1	5361.354	1	3866.951	1
303		19	max	.001	1	.01	1	0	15	-3.316e-5	15	NC	3	NC	2
304			min	0	10	-.007	3	-.013	1	-1.065e-3	1	4766.589	1	3668.942	1
305	M12	1	max	.003	1	.007	2	.01	1	9.043e-4	1	NC	1	NC	3
306			min	0	3	-.005	3	0	15	2.857e-5	15	NC	1	1857.748	1
307		2	max	.003	1	.007	2	.01	1	9.043e-4	1	NC	1	NC	3
308			min	0	3	-.005	3	0	15	2.857e-5	15	NC	1	2026.001	1
309		3	max	.002	1	.006	2	.009	1	9.043e-4	1	NC	1	NC	3
310			min	0	3	-.005	3	0	15	2.857e-5	15	NC	1	2226.28	1
311		4	max	.002	1	.006	2	.008	1	9.043e-4	1	NC	1	NC	3
312			min	0	3	-.004	3	0	15	2.857e-5	15	NC	1	2467.025	1
313		5	max	.002	1	.006	2	.007	1	9.043e-4	1	NC	1	NC	3
314			min	0	3	-.004	3	0	15	2.857e-5	15	NC	1	2759.736	1
315		6	max	.002	1	.005	2	.006	1	9.043e-4	1	NC	1	NC	3
316			min	0	3	-.004	3	0	15	2.857e-5	15	NC	1	3120.409	1
317		7	max	.002	1	.005	2	.005	1	9.043e-4	1	NC	1	NC	3
318			min	0	3	-.003	3	0	15	2.857e-5	15	NC	1	3571.826	1
319		8	max	.002	1	.004	2	.005	1	9.043e-4	1	NC	1	NC	2
320			min	0	3	-.003	3	0	15	2.857e-5	15	NC	1	4147.351	1
321		9	max	.002	1	.004	2	.004	1	9.043e-4	1	NC	1	NC	2
322			min	0	3	-.003	3	0	15	2.857e-5	15	NC	1	4897.452	1
323		10	max	.001	1	.004	2	.003	1	9.043e-4	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	2.857e-5	15	NC	1	5901.472	1
325		11	max	.001	1	.003	2	.003	1	9.043e-4	1	NC	1	NC	2
326			min	0	3	-.002	3	0	15	2.857e-5	15	NC	1	7290.106	1
327		12	max	.001	1	.003	2	.002	1	9.043e-4	1	NC	1	NC	2
328			min	0	3	-.002	3	0	15	2.857e-5	15	NC	1	9291.348	1
329		13	max	0	1	.002	2	.002	1	9.043e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	2.857e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	.001	1	9.043e-4	1	NC	1	NC	1
332			min	0	3	-.001	3	0	15	2.857e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	9.043e-4	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	2.857e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	9.043e-4	1	NC	1	NC	1
336			min	0	3	0	3	0	15	2.857e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	9.043e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	2.857e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	9.043e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	2.857e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	9.043e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	2.857e-5	15	NC	1	NC	1
343	M1	1	max	.005	3	.022	3	.001	3	1.846e-2	1	NC	1	NC	1
344			min	-.006	2	-.031	1	-.004	1	-1.508e-2	3	NC	1	NC	1
345		2	max	.005	3	.012	3	0	3	8.877e-3	1	NC	4	NC	2
346			min	-.006	2	-.016	1	-.009	1	-7.457e-3	3	3209.582	1	9498.549	1
347		3	max	.005	3	.002	3	0	3	2.15e-5	3	NC	5	NC	2
348			min	-.006	2	-.003	1	-.012	1	-5.273e-4	1	1660.183	1	5758.813	1
349		4	max	.005	3	.009	1	0	3	2.361e-5	3	NC	5	NC	2
350			min	-.006	2	-.005	3	-.014	1	-4.342e-4	1	1174.729	1	4763.709	1
351		5	max	.005	3	.019	1	0	3	2.573e-5	3	NC	5	NC	2
352			min	-.006	2	-.012	3	-.014	1	-3.41e-4	1	941.478	1	4571.914	1
353		6	max	.005	3	.027	1	0	3	2.785e-5	3	NC	5	NC	2
354			min	-.006	2	-.017	3	-.013	1	-2.479e-4	1	809.723	1	4888.103	1
355		7	max	.005	3	.033	1	0	3	2.997e-5	3	NC	5	NC	2
356			min	-.006	2	-.021	3	-.012	1	-1.547e-4	1	730.041	1	5812.401	1
357		8	max	.005	3	.037	1	0	3	3.209e-5	3	NC	5	NC	2
358			min	-.006	2	-.023	3	-.01	1	-6.16e-5	1	681.886	1	7957.696	1
359		9	max	.005	3	.04	1	0	3	3.421e-5	3	NC	5	NC	1
360			min	-.006	2	-.025	3	-.007	1	1.217e-6	15	655.672	1	NC	1
361		10	max	.005	3	.041	1	0	3	1.247e-4	1	NC	5	NC	1
362			min	-.006	2	-.025	3	-.004	1	4.071e-6	15	646.984	1	NC	1
363		11	max	.005	3	.04	1	0	3	2.178e-4	1	NC	5	NC	1
364			min	-.006	2	-.024	3	-.001	1	6.924e-6	15	654.492	1	NC	1
365		12	max	.005	3	.037	1	.002	1	3.11e-4	1	NC	5	NC	2
366			min	-.006	2	-.022	3	0	15	9.778e-6	15	679.4	1	9289.016	1
367		13	max	.005	3	.032	1	.004	1	4.041e-4	1	NC	5	NC	2
368			min	-.006	2	-.019	3	0	15	1.263e-5	15	725.953	1	6440.913	1
369		14	max	.005	3	.026	1	.006	1	4.972e-4	1	NC	5	NC	2
370			min	-.006	2	-.015	3	0	15	1.548e-5	15	803.442	1	5277.025	1
371		15	max	.005	3	.018	1	.006	1	5.904e-4	1	NC	5	NC	2
372			min	-.006	2	-.01	3	0	15	1.834e-5	15	931.777	1	4857.934	1
373		16	max	.005	3	.008	1	.006	1	6.59e-4	1	NC	5	NC	2
374			min	-.006	2	-.005	3	0	15	2.045e-5	15	1158.436	1	5004.42	1
375		17	max	.005	3	.002	3	.005	1	1.45e-4	1	NC	5	NC	2
376			min	-.006	2	-.004	1	0	15	5.032e-6	15	1626.08	1	6001.732	1
377		18	max	.005	3	.009	3	.002	1	1.035e-2	1	NC	4	NC	2
378			min	-.006	2	-.018	1	0	15	-3.564e-3	3	3134.139	1	9845.137	1
379		19	max	.005	3	.017	3	0	3	2.076e-2	1	NC	1	NC	1
380			min	-.006	2	-.033	1	-.003	1	-7.224e-3	3	NC	1	NC	1
381	M5	1	max	.014	3	.065	3	.001	3	4.519e-7	3	NC	1	NC	1
382			min	-.02	2	-.091	1	-.005	1	3.495e-8	15	NC	1	NC	1
383		2	max	.014	3	.036	3	.002	3	3.89e-5	3	NC	5	NC	1
384			min	-.02	2	-.049	1	-.004	1	-9.028e-5	1	1089.425	1	NC	1
385		3	max	.014	3	.008	3	.002	3	7.663e-5	3	NC	5	NC	1
386			min	-.02	2	-.009	1	-.004	1	-1.79e-4	1	560.991	1	NC	1
387		4	max	.014	3	.025	1	.002	3	7.602e-5	3	NC	5	NC	1
388			min	-.02	2	-.015	3	-.003	1	-1.674e-4	1	396.131	1	NC	1
389		5	max	.014	3	.055	1	.003	3	7.541e-5	3	NC	15	NC	1
390			min	-.02	2	-.034	3	-.003	1	-1.558e-4	1	316.908	1	NC	1
391		6	max	.014	3	.078	1	.003	3	7.48e-5	3	NC	15	NC	1
392			min	-.02	2	-.049	3	-.003	1	-1.442e-4	1	272.109	1	NC	1
393		7	max	.014	3	.097	1	.003	3	7.419e-5	3	NC	15	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.02	2	-.06	3	-.002	1	-1.327e-4	1	244.952	1	NC	1
395		8	max	.014	3	.11	1	.003	3	7.358e-5	3	NC	15	NC	1
396			min	-.02	2	-.067	3	-.002	1	-1.211e-4	1	228.461	1	NC	1
397		9	max	.014	3	.118	1	.003	3	7.297e-5	3	NC	15	NC	1
398			min	-.02	2	-.071	3	-.002	1	-1.095e-4	1	219.377	1	NC	1
399		10	max	.014	3	.121	1	.003	3	7.236e-5	3	NC	15	NC	1
400			min	-.02	2	-.072	3	-.002	1	-9.786e-5	1	216.193	1	NC	1
401		11	max	.014	3	.118	1	.002	3	7.175e-5	3	NC	15	NC	1
402			min	-.02	2	-.069	3	-.002	1	-8.627e-5	1	218.444	1	NC	1
403		12	max	.014	3	.11	1	.002	3	7.114e-5	3	NC	15	NC	1
404			min	-.02	2	-.063	3	-.002	1	-7.467e-5	1	226.52	1	NC	1
405		13	max	.014	3	.097	1	.002	3	7.053e-5	3	NC	15	NC	1
406			min	-.02	2	-.055	3	-.002	1	-6.308e-5	1	241.83	1	NC	1
407		14	max	.014	3	.078	1	.002	3	6.993e-5	3	NC	15	NC	1
408			min	-.02	2	-.043	3	-.002	1	-5.148e-5	1	267.475	1	NC	1
409		15	max	.014	3	.053	1	.001	3	6.932e-5	3	NC	15	NC	1
410			min	-.02	2	-.03	3	-.002	1	-3.988e-5	1	310.13	1	NC	1
411		16	max	.014	3	.023	1	0	3	6.706e-5	3	NC	5	NC	1
412			min	-.02	2	-.013	3	-.002	1	-3.459e-5	1	385.796	1	NC	1
413		17	max	.014	3	.005	3	0	3	2.566e-5	3	NC	5	NC	1
414			min	-.02	2	-.013	1	-.002	1	-1.792e-4	1	543.265	1	NC	1
415		18	max	.014	3	.026	3	0	3	1.252e-5	3	NC	5	NC	1
416			min	-.02	2	-.055	1	-.002	1	-9.182e-5	1	1052.304	1	NC	1
417		19	max	.014	3	.047	3	0	3	0	5	NC	1	NC	1
418			min	-.02	2	-.1	1	-.002	1	-8.761e-8	3	NC	1	NC	1
419	M9	1	max	.005	3	.022	3	0	3	1.508e-2	3	NC	1	NC	1
420			min	-.006	2	-.031	1	-.006	1	-1.846e-2	1	NC	1	NC	1
421		2	max	.005	3	.012	3	0	3	7.478e-3	3	NC	4	NC	1
422			min	-.006	2	-.016	1	-.001	1	-9.122e-3	1	3210.464	1	NC	1
423		3	max	.005	3	.002	3	.002	1	4.147e-5	1	NC	5	NC	2
424			min	-.006	2	-.003	1	0	3	1.502e-6	15	1660.653	1	6815.474	1
425		4	max	.005	3	.009	1	.003	1	1.091e-5	3	NC	5	NC	2
426			min	-.006	2	-.006	3	0	3	-3.544e-5	1	1175.063	1	5770.757	1
427		5	max	.005	3	.019	1	.003	1	2.334e-6	3	NC	5	NC	2
428			min	-.006	2	-.012	3	0	3	-1.123e-4	1	941.737	1	5712.276	1
429		6	max	.005	3	.026	1	.003	1	-4.287e-6	12	NC	5	NC	2
430			min	-.006	2	-.017	3	-.001	3	-1.893e-4	1	809.935	1	6397.466	1
431		7	max	.005	3	.033	1	.001	1	-8.037e-6	15	NC	5	NC	2
432			min	-.006	2	-.021	3	-.001	3	-2.662e-4	1	730.222	1	8240.264	1
433		8	max	.005	3	.037	1	0	2	-1.042e-5	15	NC	5	NC	1
434			min	-.006	2	-.024	3	-.002	3	-3.431e-4	1	682.044	1	NC	1
435		9	max	.005	3	.04	1	0	10	-1.281e-5	15	NC	5	NC	1
436			min	-.006	2	-.025	3	-.003	1	-4.2e-4	1	655.813	1	NC	1
437		10	max	.005	3	.04	1	0	15	-1.519e-5	15	NC	5	NC	1
438			min	-.006	2	-.025	3	-.005	1	-4.969e-4	1	647.112	1	NC	1
439		11	max	.005	3	.04	1	0	15	-1.758e-5	15	NC	5	NC	1
440			min	-.006	2	-.024	3	-.008	1	-5.738e-4	1	654.611	1	NC	1
441		12	max	.005	3	.037	1	0	15	-1.996e-5	15	NC	5	NC	2
442			min	-.006	2	-.022	3	-.01	1	-6.507e-4	1	679.512	1	6822.421	1
443		13	max	.005	3	.032	1	0	15	-2.234e-5	15	NC	5	NC	2
444			min	-.006	2	-.019	3	-.012	1	-7.276e-4	1	726.061	1	5296.6	1
445		14	max	.005	3	.026	1	0	15	-2.473e-5	15	NC	5	NC	2
446			min	-.006	2	-.015	3	-.013	1	-8.045e-4	1	803.547	1	4603.182	1
447		15	max	.005	3	.018	1	0	15	-2.711e-5	15	NC	5	NC	2
448			min	-.006	2	-.01	3	-.013	1	-8.814e-4	1	931.885	1	4393.418	1
449		16	max	.005	3	.008	1	0	15	-2.89e-5	15	NC	5	NC	2
450			min	-.006	2	-.005	3	-.012	1	-9.394e-4	1	1158.555	1	4636.326	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.005	3	.002	3	0	15	-1.635e-5	15	NC	5	NC	2
452			min	-.006	2	-.004	1	-.01	1	-5.482e-4	1	1626.236	1	5656.228	1
453		18	max	.005	3	.009	3	0	15	3.566e-3	3	NC	4	NC	2
454			min	-.006	2	-.018	1	-.007	1	-1.056e-2	1	3134.426	1	9396.317	1
455		19	max	.005	3	.017	3	0	3	7.224e-3	3	NC	1	NC	1
456			min	-.006	2	-.033	1	-.002	1	-2.077e-2	1	NC	1	NC	1
457	M13	1	max	.006	1	.022	3	.005	3	3.898e-3	3	NC	1	NC	1
458			min	0	3	-.031	1	-.006	2	-5.513e-3	1	NC	1	NC	1
459		2	max	.006	1	.177	3	.036	1	4.712e-3	3	NC	5	NC	2
460			min	0	3	-.221	1	0	10	-6.684e-3	1	1010.115	1	4763.947	1
461		3	max	.005	1	.304	3	.091	1	5.526e-3	3	NC	5	NC	3
462			min	0	3	-.376	1	.003	15	-7.856e-3	1	555.447	1	2000.214	1
463		4	max	.005	1	.384	3	.138	1	6.34e-3	3	NC	5	NC	3
464			min	0	3	-.474	1	.004	15	-9.027e-3	1	432.901	1	1343.47	1
465		5	max	.005	1	.407	3	.161	1	7.153e-3	3	NC	5	NC	3
466			min	0	3	-.503	1	.005	15	-1.02e-2	1	406.229	1	1159.088	1
467		6	max	.005	1	.374	3	.153	1	7.967e-3	3	NC	5	NC	3
468			min	0	3	-.465	1	.005	15	-1.137e-2	1	442.094	1	1219.157	1
469		7	max	.005	1	.298	3	.116	1	8.781e-3	3	NC	5	NC	3
470			min	0	3	-.373	1	.003	10	-1.254e-2	1	561.377	1	1594.106	1
471		8	max	.005	1	.198	3	.061	1	9.595e-3	3	NC	5	NC	2
472			min	0	3	-.252	1	-.003	10	-1.371e-2	1	866.479	1	2942.225	1
473		9	max	.005	1	.107	3	.013	3	1.041e-2	3	NC	5	NC	1
474			min	-.001	3	-.142	1	-.01	2	-1.488e-2	1	1728.906	1	NC	1
475		10	max	.005	1	.065	3	.014	3	1.122e-2	3	NC	4	NC	1
476			min	-.001	3	-.091	1	-.02	2	-1.606e-2	1	3161.948	1	NC	1
477		11	max	.005	1	.107	3	.016	3	1.041e-2	3	NC	5	NC	1
478			min	-.001	3	-.142	1	-.009	2	-1.488e-2	1	1728.906	1	NC	1
479		12	max	.005	1	.198	3	.066	1	9.595e-3	3	NC	5	NC	2
480			min	-.001	3	-.252	1	-.003	10	-1.371e-2	1	866.48	1	2722.441	1
481		13	max	.005	1	.298	3	.122	1	8.782e-3	3	NC	5	NC	3
482			min	-.001	3	-.373	1	.003	10	-1.254e-2	1	561.377	1	1515.32	1
483		14	max	.004	1	.374	3	.159	1	7.968e-3	3	NC	5	NC	3
484			min	-.001	3	-.465	1	.005	15	-1.137e-2	1	442.095	1	1171.618	1
485		15	max	.004	1	.407	3	.167	1	7.155e-3	3	NC	5	NC	3
486			min	-.001	3	-.503	1	.005	15	-1.02e-2	1	406.229	1	1119.733	1
487		16	max	.004	1	.384	3	.143	1	6.341e-3	3	NC	5	NC	3
488			min	-.001	3	-.474	1	.005	15	-9.027e-3	1	432.901	1	1300.442	1
489		17	max	.004	1	.304	3	.095	1	5.528e-3	3	NC	5	NC	3
490			min	-.001	3	-.376	1	.003	15	-7.855e-3	1	555.447	1	1934.122	1
491		18	max	.004	1	.177	3	.037	1	4.714e-3	3	NC	5	NC	2
492			min	-.001	3	-.221	1	0	10	-6.684e-3	1	1010.116	1	4576.891	1
493		19	max	.004	1	.022	3	.005	3	3.901e-3	3	NC	1	NC	1
494			min	-.001	3	-.031	1	-.006	2	-5.512e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.017	3	.005	3	5.742e-3	1	NC	1	NC	1
496			min	0	3	-.033	1	-.006	2	-2.929e-3	3	NC	1	NC	1
497		2	max	.002	1	.091	3	.038	1	6.99e-3	1	NC	5	NC	2
498			min	0	3	-.247	1	0	10	-3.512e-3	3	897.996	1	4487.668	1
499		3	max	.002	1	.152	3	.095	1	8.238e-3	1	NC	5	NC	3
500			min	0	3	-.422	1	.003	15	-4.095e-3	3	493.842	1	1915.981	1
501		4	max	.002	1	.191	3	.143	1	9.486e-3	1	NC	5	NC	3
502			min	0	3	-.532	1	.005	15	-4.678e-3	3	384.957	1	1295.771	1
503		5	max	.002	1	.203	3	.166	1	1.073e-2	1	NC	5	NC	3
504			min	0	3	-.564	1	.005	15	-5.261e-3	3	361.347	1	1120.87	1
505		6	max	.002	1	.189	3	.158	1	1.198e-2	1	NC	5	NC	3
506			min	0	3	-.521	1	.005	15	-5.844e-3	3	393.456	1	1178.554	1
507		7	max	.002	1	.155	3	.12	1	1.323e-2	1	NC	5	NC	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.417	1	.002	10	-6.427e-3	3	500.114	1	1534.995	1
509	8	max	.002	1	.109	3	.063	1	1.448e-2	1	NC	5	NC	2
510		min	0	3	-.281	1	-.003	10	-7.011e-3	3	773.641	1	2799.378	1
511	9	max	.002	1	.067	3	.016	3	1.573e-2	1	NC	5	NC	1
512		min	0	3	-.157	1	-.01	2	-7.594e-3	3	1553.041	1	NC	1
513	10	max	.002	1	.047	3	.014	3	1.697e-2	1	NC	4	NC	1
514		min	0	3	-.1	1	-.02	2	-8.177e-3	3	2868.949	1	NC	1
515	11	max	.002	1	.067	3	.014	3	1.573e-2	1	NC	5	NC	1
516		min	0	3	-.157	1	-.01	2	-7.593e-3	3	1553.041	1	NC	1
517	12	max	.003	1	.109	3	.062	1	1.448e-2	1	NC	5	NC	2
518		min	0	3	-.281	1	-.003	10	-7.01e-3	3	773.641	1	2875.936	1
519	13	max	.003	1	.155	3	.117	1	1.323e-2	1	NC	5	NC	3
520		min	0	3	-.417	1	.002	10	-6.427e-3	3	500.114	1	1567.379	1
521	14	max	.003	1	.189	3	.155	1	1.198e-2	1	NC	5	NC	3
522		min	0	3	-.521	1	.005	15	-5.843e-3	3	393.456	1	1201.676	1
523	15	max	.003	1	.203	3	.163	1	1.074e-2	1	NC	5	NC	3
524		min	0	3	-.564	1	.005	15	-5.26e-3	3	361.348	1	1143.554	1
525	16	max	.003	1	.191	3	.14	1	9.488e-3	1	NC	5	NC	3
526		min	0	3	-.532	1	.004	15	-4.676e-3	3	384.957	1	1325.24	1
527	17	max	.003	1	.152	3	.092	1	8.24e-3	1	NC	5	NC	3
528		min	0	3	-.422	1	.003	15	-4.093e-3	3	493.843	1	1969.982	1
529	18	max	.003	1	.091	3	.036	1	6.992e-3	1	NC	5	NC	2
530		min	0	3	-.247	1	0	10	-3.51e-3	3	897.997	1	4670.516	1
531	19	max	.003	1	.017	3	.005	3	5.744e-3	1	NC	1	NC	1
532		min	0	3	-.033	1	-.006	2	-2.926e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	2.827e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-8.889e-5	1	NC	1	NC	1
535	2	max	0	3	-.005	15	.001	1	7.77e-4	3	NC	5	NC	1
536		min	0	10	-.023	4	0	3	-8.394e-4	1	4381.654	4	NC	1
537	3	max	0	3	-.011	15	.004	1	1.271e-3	3	9485.355	15	NC	1
538		min	0	10	-.045	4	-.003	3	-1.59e-3	1	2229.674	4	NC	1
539	4	max	0	3	-.015	15	.008	1	1.766e-3	3	6507.507	15	NC	2
540		min	0	10	-.066	4	-.006	3	-2.341e-3	1	1529.687	4	8751.482	1
541	5	max	0	3	-.02	15	.013	1	2.26e-3	3	5077.874	15	NC	4
542		min	0	10	-.085	4	-.009	3	-3.091e-3	1	1193.63	4	5691.161	1
543	6	max	0	3	-.024	15	.019	1	2.754e-3	3	4273.568	15	NC	4
544		min	0	10	-.101	4	-.014	3	-3.842e-3	1	1004.566	4	4117.78	1
545	7	max	0	3	-.027	15	.025	1	3.249e-3	3	3789.884	15	NC	4
546		min	0	10	-.113	4	-.018	3	-4.592e-3	1	890.869	4	3204.2	1
547	8	max	0	3	-.029	15	.03	1	3.743e-3	3	3499.6	15	NC	4
548		min	0	10	-.123	4	-.022	3	-5.343e-3	1	822.633	4	2632.662	1
549	9	max	0	3	-.03	15	.036	1	4.238e-3	3	3343.353	15	NC	4
550		min	0	10	-.129	4	-.026	3	-6.093e-3	1	785.905	4	2259.793	1
551	10	max	0	3	-.031	15	.04	1	4.732e-3	3	3293.923	15	NC	4
552		min	0	10	-.131	4	-.029	3	-6.844e-3	1	774.286	4	2014.002	1
553	11	max	0	3	-.03	15	.043	1	5.226e-3	3	3343.353	15	NC	4
554		min	0	10	-.129	4	-.031	3	-7.594e-3	1	785.905	4	1857.805	1
555	12	max	0	3	-.029	15	.045	1	5.721e-3	3	3499.6	15	NC	5
556		min	0	10	-.123	4	-.033	3	-8.345e-3	1	822.633	4	1772.532	1
557	13	max	0	3	-.027	15	.044	1	6.215e-3	3	3789.884	15	NC	5
558		min	0	10	-.114	4	-.032	3	-9.095e-3	1	890.869	4	1752.484	1
559	14	max	0	3	-.024	15	.041	1	6.709e-3	3	4273.568	15	NC	5
560		min	0	10	-.101	4	-.03	3	-9.846e-3	1	1004.566	4	1804.921	1
561	15	max	0	3	-.02	15	.036	1	7.204e-3	3	5077.874	15	NC	4
562		min	0	10	-.085	4	-.026	3	-1.06e-2	1	1193.63	4	1957.558	1
563	16	max	0	3	-.016	15	.027	1	7.698e-3	3	6507.507	15	NC	4
564		min	0	10	-.067	4	-.02	3	-1.135e-2	1	1529.687	4	2286.097	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	3	-.011	15	.015	1	8.192e-3	3	9485.355	15	NC	4
566		min	0	10	-.046	4	-.011	3	-1.21e-2	1	2229.674	4	3028.382	1
567	18	max	0	3	-.006	15	.002	9	8.687e-3	3	NC	5	NC	4
568		min	0	10	-.024	4	-.005	2	-1.285e-2	1	4381.654	4	5388.01	1
569	19	max	0	3	.005	3	.015	3	9.181e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.021	2	-1.36e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.005	3	3.112e-3	3	NC	1	NC	1
572		min	0	3	-.002	1	-.007	2	-4.314e-3	1	NC	1	NC	1
573	2	max	0	10	-.005	15	.005	1	2.969e-3	3	NC	5	NC	2
574		min	0	3	-.024	4	0	10	-4.102e-3	1	4381.654	4	9679.475	1
575	3	max	0	10	-.011	15	.013	1	2.826e-3	3	9485.355	15	NC	3
576		min	0	3	-.046	4	-.004	3	-3.889e-3	1	2229.674	4	5471.243	1
577	4	max	0	10	-.016	15	.02	1	2.683e-3	3	6507.507	15	NC	4
578		min	0	3	-.066	4	-.008	3	-3.676e-3	1	1529.687	4	4156.62	1
579	5	max	0	10	-.02	15	.024	1	2.54e-3	3	5077.874	15	NC	4
580		min	0	3	-.085	4	-.01	3	-3.463e-3	1	1193.63	4	3585.22	1
581	6	max	0	10	-.024	15	.026	1	2.397e-3	3	4273.568	15	NC	4
582		min	0	3	-.101	4	-.011	3	-3.25e-3	1	1004.566	4	3333.389	1
583	7	max	0	10	-.027	15	.027	1	2.254e-3	3	3789.884	15	NC	4
584		min	0	3	-.113	4	-.012	3	-3.037e-3	1	890.869	4	3268.089	1
585	8	max	0	10	-.029	15	.027	1	2.11e-3	3	3499.6	15	NC	4
586		min	0	3	-.123	4	-.012	3	-2.825e-3	1	822.633	4	3343.384	1
587	9	max	0	10	-.03	15	.025	1	1.967e-3	3	3343.353	15	NC	4
588		min	0	3	-.128	4	-.011	3	-2.612e-3	1	785.905	4	3552.212	1
589	10	max	0	10	-.031	15	.023	1	1.824e-3	3	3293.923	15	NC	4
590		min	0	3	-.13	4	-.01	3	-2.399e-3	1	774.286	4	3914.943	1
591	11	max	0	10	-.03	15	.02	1	1.681e-3	3	3343.353	15	NC	4
592		min	0	3	-.128	4	-.009	3	-2.186e-3	1	785.905	4	4483.472	1
593	12	max	0	10	-.029	15	.017	1	1.538e-3	3	3499.6	15	NC	3
594		min	0	3	-.123	4	-.007	3	-1.973e-3	1	822.633	4	5360.864	1
595	13	max	0	10	-.027	15	.013	1	1.395e-3	3	3789.884	15	NC	2
596		min	0	3	-.113	4	-.006	3	-1.761e-3	1	890.869	4	6751.679	1
597	14	max	0	10	-.024	15	.01	1	1.252e-3	3	4273.568	15	NC	2
598		min	0	3	-.1	4	-.004	3	-1.548e-3	1	1004.566	4	9094.48	1
599	15	max	0	10	-.02	15	.006	1	1.109e-3	3	5077.874	15	NC	1
600		min	0	3	-.084	4	-.003	3	-1.335e-3	1	1193.63	4	NC	1
601	16	max	0	10	-.015	15	.003	1	9.658e-4	3	6507.507	15	NC	1
602		min	0	3	-.066	4	-.001	3	-1.122e-3	1	1529.687	4	NC	1
603	17	max	0	10	-.011	15	.001	1	8.227e-4	3	9485.355	15	NC	1
604		min	0	3	-.045	4	0	3	-9.092e-4	1	2229.674	4	NC	1
605	18	max	0	10	-.005	15	0	4	6.796e-4	3	NC	5	NC	1
606		min	0	3	-.023	4	0	2	-6.964e-4	1	4381.654	4	NC	1
607	19	max	0	1	0	1	0	1	5.365e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.071e-4	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.