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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

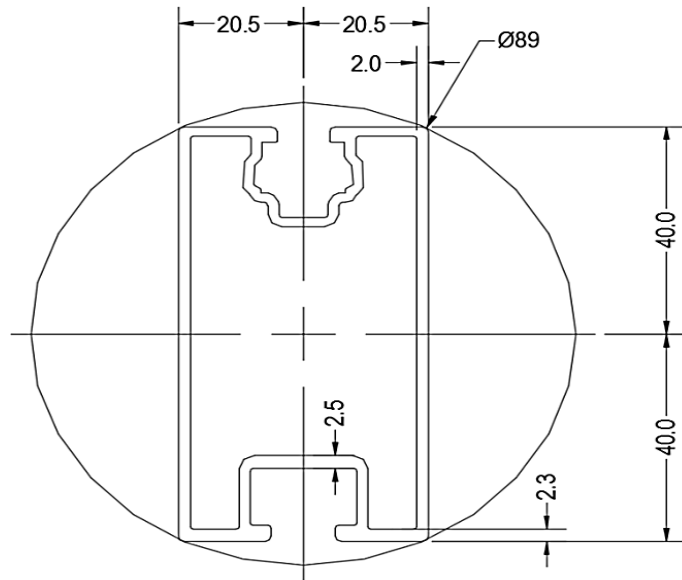
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	28.61 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	1.251 k-ft
$M_z$ =	0.235 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>98%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.82 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.609 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.240 k
$M_{y \text{ allowable}}$ =	1.463 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>43%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	1.620 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>13%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.327 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>9%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.322 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	<b>19%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.007 k-ft
$P_n$ =	0.040 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>16%</b>



A cross brace kit is required every 12 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

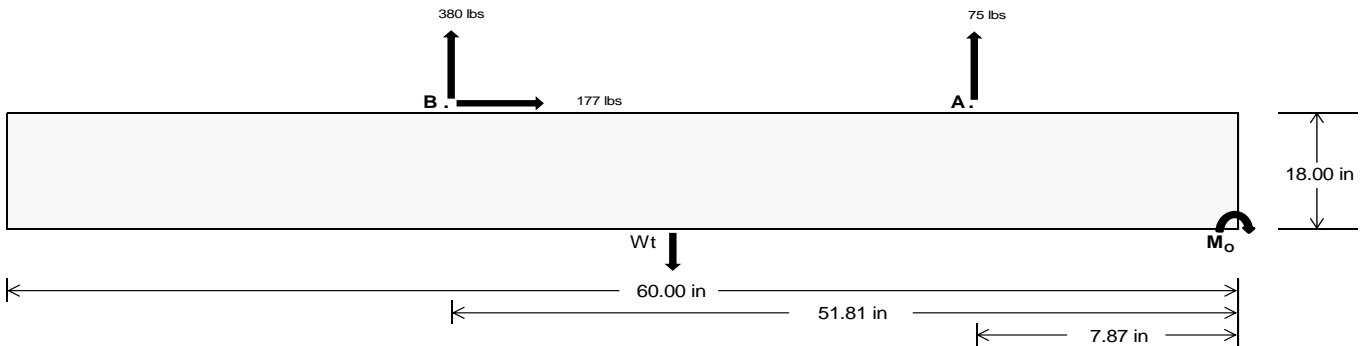
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>333.20</b>	<b>1653.89</b>	k
Compressive Load =	<b>2105.72</b>	<b>1632.85</b>	k
Lateral Load =	<b>5.33</b>	<b>768.26</b>	k
Moment (Weak Axis) =	<b>0.01</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 23466.3$  in-lbs  
Resisting Force Required = 782.21 lbs  
S.F. = 1.67  
Weight Required = 1303.69 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 177.14 lbs  
Friction = 0.4  
Weight Required = 442.85 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 177.14 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	810 lbs	810 lbs	810 lbs	810 lbs	567 lbs	567 lbs	567 lbs	567 lbs	973 lbs	973 lbs	973 lbs	973 lbs	-149 lbs	-149 lbs	-149 lbs	-149 lbs
$F_B$	595 lbs	595 lbs	595 lbs	595 lbs	499 lbs	499 lbs	499 lbs	499 lbs	775 lbs	775 lbs	775 lbs	775 lbs	-760 lbs	-760 lbs	-760 lbs	-760 lbs
$F_V$	74 lbs	74 lbs	74 lbs	74 lbs	321 lbs	321 lbs	321 lbs	321 lbs	291 lbs	291 lbs	291 lbs	291 lbs	-354 lbs	-354 lbs	-354 lbs	-354 lbs
$P_{total}$	3399 lbs	3490 lbs	3580 lbs	3671 lbs	3059 lbs	3150 lbs	3240 lbs	3331 lbs	3741 lbs	3832 lbs	3922 lbs	4013 lbs	287 lbs	341 lbs	396 lbs	450 lbs
$M$	524 lbs-ft	524 lbs-ft	524 lbs-ft	524 lbs-ft	619 lbs-ft	619 lbs-ft	619 lbs-ft	619 lbs-ft	822 lbs-ft	822 lbs-ft	822 lbs-ft	822 lbs-ft	575 lbs-ft	575 lbs-ft	575 lbs-ft	575 lbs-ft
$e$	0.15 ft	0.15 ft	0.15 ft	0.14 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	2.00 ft	1.68 ft	1.45 ft	1.28 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	302.2 psf	298.5 psf	295.2 psf	292.1 psf	252.6 psf	251.1 psf	249.7 psf	248.4 psf	300.6 psf	296.9 psf	293.6 psf	290.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	439.4 psf	429.7 psf	420.9 psf	412.7 psf	414.8 psf	406.2 psf	398.3 psf	391.1 psf	515.7 psf	502.7 psf	490.8 psf	479.9 psf	210.2 psf	145.5 psf	126.0 psf	117.8 psf

Maximum Bearing Pressure = 516 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

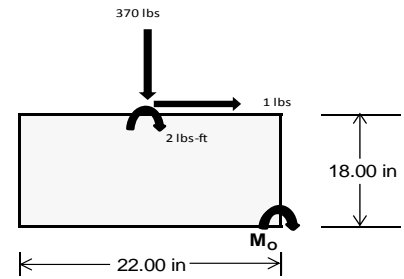
## Overturning Check

$M_o = 335.5 \text{ ft-lbs}$   
 Resisting Force Required = 366.03 lbs  
 S.F. = 1.67  
 Weight Required = 610.04 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	94 lbs	261 lbs	89 lbs	375 lbs	1144 lbs	370 lbs	28 lbs	76 lbs	26 lbs
$F_v$	5 lbs	5 lbs	0 lbs	24 lbs	22 lbs	1 lbs	1 lbs	1 lbs	0 lbs
$P_{total}$	2563 lbs	2729 lbs	2557 lbs	2725 lbs	3494 lbs	2720 lbs	749 lbs	798 lbs	748 lbs
$M$	7 lbs-ft	7 lbs-ft	0 lbs-ft	40 lbs-ft	34 lbs-ft	4 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.80 ft	1.81 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	276.9 sqft	295.2 sqft	278.9 sqft	282.8 sqft	369.1 sqft	295.4 sqft	81.0 sqft	86.3 sqft	81.5 sqft
$f_{max}$	282.2 psf	300.3 psf	279.1 psf	311.7 psf	393.2 psf	298.0 psf	82.5 psf	87.8 psf	81.6 psf



Maximum Bearing Pressure = 393 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



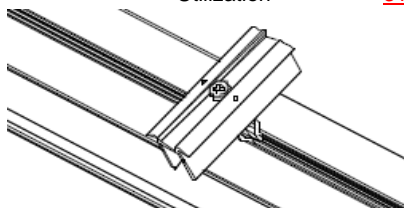
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

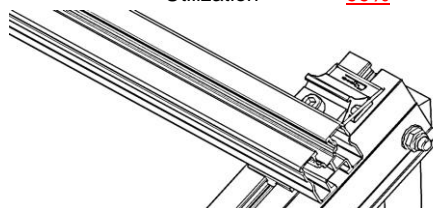
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.372 k
Allowable Uplift =	1.214 k
Utilization =	<u>31%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.106 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

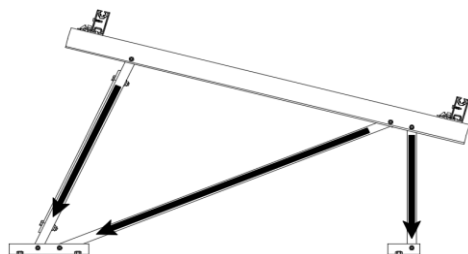
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.620 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>28%</u>

#### Diagonal Strut

Maximum Axial Load =	0.327 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



#### Rear Strut

Maximum Axial Load =	1.322 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

#### Bracing

Maximum Axial Load =	0.040 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	29.57 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.591 in
Max Drift, $\Delta_{MAX}$ =	0.068 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &21.0529 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.36 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max St} = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max Wk} = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	135.808	2	341.982	1	.002	10	0	2	0	1	0	1
2		min	-184.889	3	-386.919	3	-.195	1	0	3	0	1	0	1
3	N7	max	0	15	568.023	1	-.072	15	0	15	0	1	0	1
4		min	-.204	1	-69.173	3	-1.861	1	-.003	1	0	1	0	1
5	N15	max	-.001	15	1619.786	1	.616	1	.001	1	0	1	0	1
6		min	-2.186	1	-256.305	3	-.255	3	0	3	0	1	0	1
7	N16	max	561.79	2	1256.042	1	-.289	10	0	1	0	1	0	1
8		min	-590.97	3	-1272.224	3	-37.617	1	0	3	0	1	0	1
9	N23	max	0	15	567.799	1	4.098	1	.007	1	0	1	0	1
10		min	-.204	1	-68.721	3	.149	15	0	15	0	1	0	1
11	N24	max	136.31	2	347.831	1	34.96	1	.002	1	0	1	0	1
12		min	-184.941	3	-383.851	3	.051	10	0	3	0	1	0	1
13	Totals:	max	831.724	2	4701.464	1	0	3						
14		min	-961.105	3	-2437.193	3	0	1						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	402.095	1	.642	4	.872	1	0	15	0	3	0	1
2			min	-366.236	3	.152	15	-.036	3	-.001	1	0	1	0	1
3		2	max	402.202	1	.601	4	.872	1	0	15	0	1	0	15
4			min	-366.156	3	.142	15	-.036	3	-.001	1	0	10	0	4
5		3	max	402.308	1	.56	4	.872	1	0	15	0	1	0	15
6			min	-366.077	3	.133	15	-.036	3	-.001	1	0	12	0	4
7		4	max	402.415	1	.519	4	.872	1	0	15	0	1	0	15
8			min	-365.997	3	.123	15	-.036	3	-.001	1	0	3	0	4
9		5	max	402.521	1	.477	4	.872	1	0	15	0	1	0	15
10			min	-365.917	3	.113	15	-.036	3	-.001	1	0	3	0	4
11		6	max	402.628	1	.436	4	.872	1	0	15	0	1	0	15
12			min	-365.837	3	.104	15	-.036	3	-.001	1	0	3	0	4
13		7	max	402.734	1	.395	4	.872	1	0	15	0	1	0	15
14			min	-365.757	3	.094	15	-.036	3	-.001	1	0	3	0	4
15		8	max	402.841	1	.354	4	.872	1	0	15	0	1	0	15
16			min	-365.677	3	.084	15	-.036	3	-.001	1	0	3	0	4
17		9	max	402.948	1	.312	4	.872	1	0	15	0	1	0	15
18			min	-365.597	3	.075	15	-.036	3	-.001	1	0	3	0	4
19		10	max	403.054	1	.271	4	.872	1	0	15	.001	1	0	15
20			min	-365.517	3	.065	15	-.036	3	-.001	1	0	3	0	4
21		11	max	403.161	1	.23	4	.872	1	0	15	.001	1	0	15
22			min	-365.437	3	.055	15	-.036	3	-.001	1	0	3	0	4
23		12	max	403.267	1	.189	4	.872	1	0	15	.001	1	0	15
24			min	-365.357	3	.045	15	-.036	3	-.001	1	0	3	0	4
25		13	max	403.374	1	.147	4	.872	1	0	15	.002	1	0	15
26			min	-365.277	3	.036	15	-.036	3	-.001	1	0	3	0	4
27		14	max	403.48	1	.106	4	.872	1	0	15	.002	1	0	15
28			min	-365.198	3	.026	15	-.036	3	-.001	1	0	3	0	4
29		15	max	403.587	1	.071	2	.872	1	0	15	.002	1	0	15
30			min	-365.118	3	.014	12	-.036	3	-.001	1	0	3	0	4
31		16	max	403.693	1	.039	2	.872	1	0	15	.002	1	0	15
32			min	-365.038	3	-.006	9	-.036	3	-.001	1	0	3	0	4
33		17	max	403.8	1	.009	10	.872	1	0	15	.002	1	0	15
34			min	-364.958	3	-.034	1	-.036	3	-.001	1	0	3	0	4
35		18	max	403.906	1	-.013	15	.872	1	0	15	.002	1	0	15
36			min	-364.878	3	-.066	1	-.036	3	-.001	1	0	3	0	4
37		19	max	404.013	1	-.022	15	.872	1	0	15	.002	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-364.798	3	-.1	4	-.036	3	-.001	1	0	3	0	4
39	M3	1	max	60.058	2	1.795	4	-.028	15	0	.003	1	0	4
40		min	-108.672	9	.423	15	-.847	1	0	1	0	15	0	15
41		2	max	59.991	2	1.617	4	-.028	15	0	.002	1	0	4
42		min	-108.729	9	.381	15	-.847	1	0	1	0	15	0	15
43		3	max	59.923	2	1.439	4	-.028	15	0	.002	1	0	2
44		min	-108.786	9	.339	15	-.847	1	0	1	0	15	0	9
45		4	max	59.855	2	1.262	4	-.028	15	0	.002	1	0	15
46		min	-108.842	9	.297	15	-.847	1	0	1	0	15	0	4
47		5	max	59.787	2	1.084	4	-.028	15	0	.002	1	0	15
48		min	-108.899	9	.256	15	-.847	1	0	1	0	15	0	4
49		6	max	59.719	2	.906	4	-.028	15	0	.002	1	0	15
50		min	-108.955	9	.214	15	-.847	1	0	1	0	15	0	4
51		7	max	59.651	2	.729	4	-.028	15	0	.001	1	0	15
52		min	-109.012	9	.172	15	-.847	1	0	1	0	15	0	4
53		8	max	59.583	2	.551	4	-.028	15	0	.001	1	0	15
54		min	-109.068	9	.13	15	-.847	1	0	1	0	15	0	4
55		9	max	59.516	2	.373	4	-.028	15	0	.001	1	0	15
56		min	-109.125	9	.089	15	-.847	1	0	1	0	15	-.001	4
57		10	max	59.448	2	.196	4	-.028	15	0	0	1	0	15
58		min	-109.181	9	.047	15	-.847	1	0	1	0	15	-.001	4
59		11	max	59.38	2	.029	2	-.028	15	0	0	1	0	15
60		min	-109.238	9	-.003	9	-.847	1	0	1	0	15	-.001	4
61		12	max	59.312	2	-.037	15	-.028	15	0	0	1	0	15
62		min	-109.295	9	-.16	4	-.847	1	0	1	0	15	-.001	4
63		13	max	59.244	2	-.078	15	-.028	15	0	0	1	0	15
64		min	-109.351	9	-.337	4	-.847	1	0	1	0	12	-.001	4
65		14	max	59.176	2	-.12	15	-.028	15	0	0	1	0	15
66		min	-109.408	9	-.515	4	-.847	1	0	1	0	12	-.001	4
67		15	max	59.108	2	-.162	15	-.028	15	0	0	1	0	15
68		min	-109.464	9	-.693	4	-.847	1	0	1	0	3	0	4
69		16	max	59.041	2	-.204	15	-.028	15	0	0	15	0	15
70		min	-109.521	9	-.87	4	-.847	1	0	1	0	1	0	4
71		17	max	58.973	2	-.246	15	-.028	15	0	0	15	0	15
72		min	-109.577	9	-1.048	4	-.847	1	0	1	0	1	0	4
73		18	max	58.905	2	-.287	15	-.028	15	0	0	15	0	15
74		min	-109.634	9	-1.226	4	-.847	1	0	1	0	1	0	4
75		19	max	58.837	2	-.329	15	-.028	15	0	0	15	0	1
76		min	-109.69	9	-1.403	4	-.847	1	0	1	0	1	0	1
77	M4	1	max	566.859	1	0	1	-.072	15	0	0	3	0	1
78		min	-70.047	3	0	1	-2.055	1	0	1	0	1	0	1
79		2	max	566.923	1	0	1	-.072	15	0	0	12	0	1
80		min	-69.998	3	0	1	-2.055	1	0	1	0	1	0	1
81		3	max	566.988	1	0	1	-.072	15	0	0	15	0	1
82		min	-69.95	3	0	1	-2.055	1	0	1	0	1	0	1
83		4	max	567.053	1	0	1	-.072	15	0	0	15	0	1
84		min	-69.901	3	0	1	-2.055	1	0	1	0	1	0	1
85		5	max	567.118	1	0	1	-.072	15	0	0	15	0	1
86		min	-69.852	3	0	1	-2.055	1	0	1	0	1	0	1
87		6	max	567.182	1	0	1	-.072	15	0	0	15	0	1
88		min	-69.804	3	0	1	-2.055	1	0	1	0	1	0	1
89		7	max	567.247	1	0	1	-.072	15	0	0	15	0	1
90		min	-69.755	3	0	1	-2.055	1	0	1	-.001	1	0	1
91		8	max	567.312	1	0	1	-.072	15	0	0	15	0	1
92		min	-69.707	3	0	1	-2.055	1	0	1	-.001	1	0	1
93		9	max	567.376	1	0	1	-.072	15	0	0	15	0	1
94		min	-69.658	3	0	1	-2.055	1	0	1	-.002	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	567.441	1	0	1	-.072	15	0	1	0	15	0	1
96		min	-69.61	3	0	1	-2.055	1	0	1	-.002	1	0	1
97	11	max	567.506	1	0	1	-.072	15	0	1	0	15	0	1
98		min	-69.561	3	0	1	-2.055	1	0	1	-.002	1	0	1
99	12	max	567.57	1	0	1	-.072	15	0	1	0	15	0	1
100		min	-69.513	3	0	1	-2.055	1	0	1	-.002	1	0	1
101	13	max	567.635	1	0	1	-.072	15	0	1	0	15	0	1
102		min	-69.464	3	0	1	-2.055	1	0	1	-.002	1	0	1
103	14	max	567.7	1	0	1	-.072	15	0	1	0	15	0	1
104		min	-69.416	3	0	1	-2.055	1	0	1	-.002	1	0	1
105	15	max	567.765	1	0	1	-.072	15	0	1	0	15	0	1
106		min	-69.367	3	0	1	-2.055	1	0	1	-.003	1	0	1
107	16	max	567.829	1	0	1	-.072	15	0	1	0	15	0	1
108		min	-69.319	3	0	1	-2.055	1	0	1	-.003	1	0	1
109	17	max	567.894	1	0	1	-.072	15	0	1	0	15	0	1
110		min	-69.27	3	0	1	-2.055	1	0	1	-.003	1	0	1
111	18	max	567.959	1	0	1	-.072	15	0	1	0	15	0	1
112		min	-69.222	3	0	1	-2.055	1	0	1	-.003	1	0	1
113	19	max	568.023	1	0	1	-.072	15	0	1	0	15	0	1
114		min	-69.173	3	0	1	-2.055	1	0	1	-.003	1	0	1
115	M6	1	max 1320.16	1	.637	4	.288	1	0	1	0	3	0	1
116		min	-1200.777	3	.151	15	-.112	3	0	15	0	2	0	1
117	2	max	1320.266	1	.595	4	.288	1	0	1	0	3	0	15
118		min	-1200.698	3	.142	15	-.112	3	0	15	0	2	0	4
119	3	max	1320.373	1	.554	4	.288	1	0	1	0	1	0	15
120		min	-1200.618	3	.132	15	-.112	3	0	15	0	15	0	4
121	4	max	1320.48	1	.513	4	.288	1	0	1	0	1	0	15
122		min	-1200.538	3	.122	15	-.112	3	0	15	0	12	0	4
123	5	max	1320.586	1	.471	4	.288	1	0	1	0	1	0	15
124		min	-1200.458	3	.112	15	-.112	3	0	15	0	3	0	4
125	6	max	1320.693	1	.43	4	.288	1	0	1	0	1	0	15
126		min	-1200.378	3	.103	15	-.112	3	0	15	0	3	0	4
127	7	max	1320.799	1	.389	4	.288	1	0	1	0	1	0	15
128		min	-1200.298	3	.093	15	-.112	3	0	15	0	3	0	4
129	8	max	1320.906	1	.348	4	.288	1	0	1	0	1	0	15
130		min	-1200.218	3	.083	15	-.112	3	0	15	0	3	0	4
131	9	max	1321.012	1	.315	2	.288	1	0	1	0	1	0	15
132		min	-1200.138	3	.074	15	-.112	3	0	15	0	3	0	4
133	10	max	1321.119	1	.283	2	.288	1	0	1	0	1	0	15
134		min	-1200.058	3	.064	15	-.112	3	0	15	0	3	0	4
135	11	max	1321.225	1	.251	2	.288	1	0	1	0	1	0	15
136		min	-1199.978	3	.052	12	-.112	3	0	15	0	3	0	4
137	12	max	1321.332	1	.218	2	.288	1	0	1	0	1	0	15
138		min	-1199.899	3	.036	12	-.112	3	0	15	0	3	0	4
139	13	max	1321.438	1	.186	2	.288	1	0	1	0	1	0	15
140		min	-1199.819	3	.02	12	-.112	3	0	15	0	3	0	4
141	14	max	1321.545	1	.154	2	.288	1	0	1	0	1	0	15
142		min	-1199.739	3	-.001	3	-.112	3	0	15	0	3	0	4
143	15	max	1321.652	1	.122	2	.288	1	0	1	0	1	0	15
144		min	-1199.659	3	-.025	3	-.112	3	0	15	0	3	0	4
145	16	max	1321.758	1	.09	2	.288	1	0	1	0	1	0	15
146		min	-1199.579	3	-.05	3	-.112	3	0	15	0	3	0	2
147	17	max	1321.865	1	.058	2	.288	1	0	1	0	1	0	15
148		min	-1199.499	3	-.074	3	-.112	3	0	15	0	3	0	2
149	18	max	1321.971	1	.026	2	.288	1	0	1	0	1	0	15
150		min	-1199.419	3	-.098	3	-.112	3	0	15	0	3	0	2
151	19	max	1322.078	1	-.007	2	.288	1	0	1	0	1	0	15





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1199.339	3	-.122	3	-.112	3	0	15	0	3	0	2
153	M7	1	max	327.174	2	1.793	4	.018	1	0	2	0	2	2
154		min	-267.715	3	.423	15	-.002	3	0	3	0	3	0	12
155		2	max	327.106	2	1.616	4	.018	1	0	2	0	2	2
156		min	-267.766	3	.381	15	-.002	3	0	3	0	3	0	3
157		3	max	327.038	2	1.438	4	.018	1	0	2	0	2	2
158		min	-267.817	3	.339	15	-.002	3	0	3	0	3	0	3
159		4	max	326.971	2	1.26	4	.018	1	0	2	0	2	2
160		min	-267.868	3	.297	15	-.002	3	0	3	0	3	0	3
161		5	max	326.903	2	1.083	4	.018	1	0	2	0	2	15
162		min	-267.919	3	.255	15	-.002	3	0	3	0	3	0	4
163		6	max	326.835	2	.905	4	.018	1	0	2	0	2	15
164		min	-267.969	3	.214	15	-.002	3	0	3	0	3	0	4
165		7	max	326.767	2	.727	4	.018	1	0	2	0	2	15
166		min	-268.02	3	.172	15	-.002	3	0	3	0	3	0	4
167		8	max	326.699	2	.55	4	.018	1	0	2	0	2	15
168		min	-268.071	3	.13	15	-.002	3	0	3	0	3	-.001	4
169		9	max	326.631	2	.372	4	.018	1	0	2	0	2	15
170		min	-268.122	3	.088	15	-.002	3	0	3	0	3	-.001	4
171		10	max	326.563	2	.216	2	.018	1	0	2	0	2	15
172		min	-268.173	3	.043	12	-.002	3	0	3	0	3	-.001	4
173		11	max	326.495	2	.078	2	.018	1	0	2	0	2	15
174		min	-268.224	3	-.046	3	-.002	3	0	3	0	3	-.001	4
175		12	max	326.428	2	-.037	15	.018	1	0	2	0	2	15
176		min	-268.275	3	-.161	4	-.002	3	0	3	0	3	-.001	4
177		13	max	326.36	2	-.079	15	.018	1	0	2	0	2	15
178		min	-268.326	3	-.338	4	-.002	3	0	3	0	3	-.001	4
179		14	max	326.292	2	-.12	15	.018	1	0	2	0	2	15
180		min	-268.377	3	-.516	4	-.002	3	0	3	0	3	-.001	4
181		15	max	326.224	2	-.162	15	.018	1	0	2	0	2	15
182		min	-268.428	3	-.694	4	-.002	3	0	3	0	3	0	4
183		16	max	326.156	2	-.204	15	.018	1	0	2	0	2	15
184		min	-268.478	3	-.871	4	-.002	3	0	3	0	3	0	4
185		17	max	326.088	2	-.246	15	.018	1	0	2	0	2	15
186		min	-268.529	3	-1.049	4	-.002	3	0	3	0	3	0	4
187		18	max	326.02	2	-.287	15	.018	1	0	2	0	2	15
188		min	-268.58	3	-1.227	4	-.002	3	0	3	0	3	0	4
189		19	max	325.953	2	-.329	15	.018	1	0	2	0	2	1
190		min	-268.631	3	-1.404	4	-.002	3	0	3	0	3	0	1
191	M8	1	max	1618.622	1	0	1	.837	1	0	1	0	15	0
192		min	-257.179	3	0	1	-.245	3	0	1	0	1	0	1
193		2	max	1618.686	1	0	1	.837	1	0	1	0	1	0
194		min	-257.13	3	0	1	-.245	3	0	1	0	3	0	1
195		3	max	1618.751	1	0	1	.837	1	0	1	0	1	0
196		min	-257.082	3	0	1	-.245	3	0	1	0	3	0	1
197		4	max	1618.816	1	0	1	.837	1	0	1	0	1	0
198		min	-257.033	3	0	1	-.245	3	0	1	0	3	0	1
199		5	max	1618.881	1	0	1	.837	1	0	1	0	1	0
200		min	-256.985	3	0	1	-.245	3	0	1	0	3	0	1
201		6	max	1618.945	1	0	1	.837	1	0	1	0	1	0
202		min	-256.936	3	0	1	-.245	3	0	1	0	3	0	1
203		7	max	1619.01	1	0	1	.837	1	0	1	0	1	0
204		min	-256.888	3	0	1	-.245	3	0	1	0	3	0	1
205		8	max	1619.075	1	0	1	.837	1	0	1	0	1	0
206		min	-256.839	3	0	1	-.245	3	0	1	0	3	0	1
207		9	max	1619.139	1	0	1	.837	1	0	1	0	1	0
208		min	-256.79	3	0	1	-.245	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1619.204	1	0	1	.837	1	0	1	0	1	0	1
210			min	-256.742	3	0	1	-.245	3	0	1	0	3	0	1
211		11	max	1619.269	1	0	1	.837	1	0	1	0	1	0	1
212			min	-256.693	3	0	1	-.245	3	0	1	0	3	0	1
213		12	max	1619.334	1	0	1	.837	1	0	1	0	1	0	1
214			min	-256.645	3	0	1	-.245	3	0	1	0	3	0	1
215		13	max	1619.398	1	0	1	.837	1	0	1	0	1	0	1
216			min	-256.596	3	0	1	-.245	3	0	1	0	3	0	1
217		14	max	1619.463	1	0	1	.837	1	0	1	0	1	0	1
218			min	-256.548	3	0	1	-.245	3	0	1	0	3	0	1
219		15	max	1619.528	1	0	1	.837	1	0	1	.001	1	0	1
220			min	-256.499	3	0	1	-.245	3	0	1	0	3	0	1
221		16	max	1619.592	1	0	1	.837	1	0	1	.001	1	0	1
222			min	-256.451	3	0	1	-.245	3	0	1	0	3	0	1
223		17	max	1619.657	1	0	1	.837	1	0	1	.001	1	0	1
224			min	-256.402	3	0	1	-.245	3	0	1	0	3	0	1
225		18	max	1619.722	1	0	1	.837	1	0	1	.001	1	0	1
226			min	-256.354	3	0	1	-.245	3	0	1	0	3	0	1
227		19	max	1619.786	1	0	1	.837	1	0	1	.001	1	0	1
228			min	-256.305	3	0	1	-.245	3	0	1	0	3	0	1
229	M10	1	max	416.596	1	.633	4	-.007	15	.001	1	0	1	0	1
230			min	-354.811	3	.151	15	-.191	1	0	3	0	3	0	1
231		2	max	416.703	1	.591	4	-.007	15	.001	1	0	1	0	15
232			min	-354.731	3	.141	15	-.191	1	0	3	0	3	0	4
233		3	max	416.81	1	.55	4	-.007	15	.001	1	0	1	0	15
234			min	-354.651	3	.131	15	-.191	1	0	3	0	3	0	4
235		4	max	416.916	1	.509	4	-.007	15	.001	1	0	1	0	15
236			min	-354.571	3	.122	15	-.191	1	0	3	0	3	0	4
237		5	max	417.023	1	.467	4	-.007	15	.001	1	0	2	0	15
238			min	-354.491	3	.112	15	-.191	1	0	3	0	3	0	4
239		6	max	417.129	1	.426	4	-.007	15	.001	1	0	2	0	15
240			min	-354.411	3	.102	15	-.191	1	0	3	0	3	0	4
241		7	max	417.236	1	.385	4	-.007	15	.001	1	0	2	0	15
242			min	-354.331	3	.093	15	-.191	1	0	3	0	3	0	4
243		8	max	417.342	1	.344	4	-.007	15	.001	1	0	2	0	15
244			min	-354.251	3	.083	15	-.191	1	0	3	0	3	0	4
245		9	max	417.449	1	.302	4	-.007	15	.001	1	0	2	0	15
246			min	-354.172	3	.073	15	-.191	1	0	3	0	1	0	4
247		10	max	417.555	1	.261	4	-.007	15	.001	1	0	15	0	15
248			min	-354.092	3	.063	15	-.191	1	0	3	0	1	0	4
249		11	max	417.662	1	.22	4	-.007	15	.001	1	0	15	0	15
250			min	-354.012	3	.054	15	-.191	1	0	3	0	1	0	4
251		12	max	417.768	1	.179	4	-.007	15	.001	1	0	15	0	15
252			min	-353.932	3	.044	15	-.191	1	0	3	0	1	0	4
253		13	max	417.875	1	.137	4	-.007	15	.001	1	0	15	0	15
254			min	-353.852	3	.034	15	-.191	1	0	3	0	1	0	4
255		14	max	417.981	1	.096	4	-.007	15	.001	1	0	15	0	15
256			min	-353.772	3	.013	1	-.191	1	0	3	0	1	0	4
257		15	max	418.088	1	.062	10	-.007	15	.001	1	0	15	0	15
258			min	-353.692	3	-.019	1	-.191	1	0	3	0	1	0	4
259		16	max	418.195	1	.036	10	-.007	15	.001	1	0	15	0	15
260			min	-353.612	3	-.051	1	-.191	1	0	3	0	1	0	4
261		17	max	418.301	1	.009	10	-.007	15	.001	1	0	15	0	15
262			min	-353.532	3	-.083	1	-.191	1	0	3	0	1	0	4
263		18	max	418.408	1	-.014	15	-.007	15	.001	1	0	15	0	15
264			min	-353.452	3	-.115	1	-.191	1	0	3	0	1	0	4
265		19	max	418.514	1	-.024	15	-.007	15	.001	1	0	15	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	1	min	-353.372	3	-.148	1	-.191	1	0	3	0	1	0	4
267		1	max	59.801	2	1.8	4	.98	1	.001	1	0	3	0	4
268			min	-108.584	9	.423	15	.023	12	0	15	-.002	1	0	15
269		2	max	59.733	2	1.622	4	.98	1	.001	1	0	3	0	2
270			min	-108.64	9	.382	15	.023	12	0	15	-.002	1	0	12
271		3	max	59.666	2	1.444	4	.98	1	.001	1	0	3	0	2
272			min	-108.697	9	.34	15	.023	12	0	15	-.002	1	0	3
273		4	max	59.598	2	1.267	4	.98	1	.001	1	0	3	0	15
274			min	-108.753	9	.298	15	.023	12	0	15	-.002	1	0	3
275		5	max	59.53	2	1.089	4	.98	1	.001	1	0	3	0	15
276			min	-108.81	9	.256	15	.023	12	0	15	-.002	1	0	4
277		6	max	59.462	2	.911	4	.98	1	.001	1	0	3	0	15
278			min	-108.867	9	.215	15	.023	12	0	15	-.001	1	0	4
279		7	max	59.394	2	.734	4	.98	1	.001	1	0	3	0	15
280			min	-108.923	9	.173	15	.023	12	0	15	-.001	1	0	4
281		8	max	59.326	2	.556	4	.98	1	.001	1	0	3	0	15
282			min	-108.98	9	.131	15	.023	12	0	15	-.001	1	0	4
283	9	max	59.258	2	.378	4	.98	1	.001	1	0	3	0	15	
284		min	-109.036	9	.089	15	.023	12	0	15	0	1	-.001	4	
285	10	max	59.191	2	.201	4	.98	1	.001	1	0	3	0	15	
286		min	-109.093	9	.048	15	.023	12	0	15	0	1	-.001	4	
287	11	max	59.123	2	.05	2	.98	1	.001	1	0	3	0	15	
288		min	-109.149	9	-.021	3	.023	12	0	15	0	1	-.001	4	
289	12	max	59.055	2	-.036	15	.98	1	.001	1	0	3	0	15	
290		min	-109.206	9	-.155	4	.023	12	0	15	0	1	-.001	4	
291	13	max	58.987	2	-.078	15	.98	1	.001	1	0	3	0	15	
292		min	-109.262	9	-.332	4	.023	12	0	15	0	2	-.001	4	
293	14	max	58.919	2	-.12	15	.98	1	.001	1	0	1	0	15	
294		min	-109.319	9	-.51	4	.023	12	0	15	0	10	-.001	4	
295	15	max	58.851	2	-.161	15	.98	1	.001	1	0	1	0	15	
296		min	-109.375	9	-.688	4	.023	12	0	15	0	15	0	4	
297	16	max	58.783	2	-.203	15	.98	1	.001	1	0	1	0	15	
298		min	-109.432	9	-.865	4	.023	12	0	15	0	15	0	4	
299	17	max	58.715	2	-.245	15	.98	1	.001	1	0	1	0	15	
300		min	-109.489	9	-1.043	4	.023	12	0	15	0	15	0	4	
301	18	max	58.648	2	-.287	15	.98	1	.001	1	.001	1	0	15	
302		min	-109.545	9	-1.22	4	.023	12	0	15	0	15	0	4	
303	19	max	58.58	2	-.328	15	.98	1	.001	1	.001	1	0	1	
304		min	-109.602	9	-1.398	4	.023	12	0	15	0	15	0	1	
305	M12	1	max	566.634	1	0	1	4.52	1	0	1	0	1	0	1
306		min	-69.595	3	0	1	.15	15	0	1	0	3	0	1	
307	2	max	566.699	1	0	1	4.52	1	0	1	0	1	0	1	
308		min	-69.546	3	0	1	.15	15	0	1	0	15	0	1	
309	3	max	566.763	1	0	1	4.52	1	0	1	0	1	0	1	
310		min	-69.498	3	0	1	.15	15	0	1	0	15	0	1	
311	4	max	566.828	1	0	1	4.52	1	0	1	.001	1	0	1	
312		min	-69.449	3	0	1	.15	15	0	1	0	15	0	1	
313	5	max	566.893	1	0	1	4.52	1	0	1	.002	1	0	1	
314		min	-69.401	3	0	1	.15	15	0	1	0	15	0	1	
315	6	max	566.957	1	0	1	4.52	1	0	1	.002	1	0	1	
316		min	-69.352	3	0	1	.15	15	0	1	0	15	0	1	
317	7	max	567.022	1	0	1	4.52	1	0	1	.002	1	0	1	
318		min	-69.303	3	0	1	.15	15	0	1	0	15	0	1	
319	8	max	567.087	1	0	1	4.52	1	0	1	.003	1	0	1	
320		min	-69.255	3	0	1	.15	15	0	1	0	15	0	1	
321	9	max	567.151	1	0	1	4.52	1	0	1	.003	1	0	1	
322		min	-69.206	3	0	1	.15	15	0	1	0	15	0	1	





Company : Schletter, Inc.  
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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-143.178	1	-154.94	3	-91.328	1	0	1	-.174	1	-.007	3
381	M5	1	max	314.366	1	1139.817	3	-.109	10	0	.006	1	.02	3
382		min	9.412	12	-1328.628	1	-32.003	1	0	3	0	10	-.027	1
383		2	max	314.461	1	1139.62	3	-.109	10	0	0	2	.261	1
384		min	9.46	12	-1328.89	1	-32.003	1	0	3	-.003	3	-.227	3
385		3	max	241.814	1	9.388	9	3.065	3	0	0	10	.543	1
386		min	7.597	10	-71.065	3	-.127	2	0	1	-.008	3	-.469	3
387		4	max	241.91	1	9.169	9	3.065	3	0	0	10	.548	1
388		min	7.676	10	-71.262	3	-.127	2	0	1	-.008	3	-.454	3
389		5	max	242.005	1	8.951	9	3.065	3	0	0	10	.552	1
390		min	7.756	10	-71.459	3	-.127	2	0	1	-.007	3	-.438	3
391		6	max	242.101	1	8.732	9	3.065	3	0	0	10	.557	1
392		min	7.835	10	-71.655	3	-.127	2	0	1	-.006	1	-.423	3
393		7	max	242.196	1	8.513	9	3.065	3	0	0	10	.561	1
394		min	7.915	10	-71.852	3	-.127	2	0	1	-.006	1	-.407	3
395		8	max	242.292	1	8.295	9	3.065	3	0	0	10	.566	1
396		min	7.995	10	-72.049	3	-.127	2	0	1	-.005	1	-.392	3
397		9	max	242.387	1	8.076	9	3.065	3	0	0	10	.57	1
398		min	8.074	10	-72.246	3	-.127	2	0	1	-.005	1	-.376	3
399		10	max	242.483	1	7.857	9	3.065	3	0	0	10	.575	1
400		min	8.154	10	-72.443	3	-.127	2	0	1	-.004	1	-.36	3
401		11	max	242.578	1	7.639	9	3.065	3	0	0	10	.58	1
402		min	8.233	10	-72.639	3	-.127	2	0	1	-.004	1	-.345	3
403		12	max	242.674	1	7.42	9	3.065	3	0	0	10	.584	1
404		min	8.313	10	-72.836	3	-.127	2	0	1	-.003	1	-.329	3
405		13	max	242.769	1	7.201	9	3.065	3	0	0	10	.589	1
406		min	8.392	10	-73.033	3	-.127	2	0	1	-.003	1	-.313	3
407		14	max	242.865	1	6.983	9	3.065	3	0	0	15	.594	1
408		min	8.472	10	-73.23	3	-.127	2	0	1	-.002	1	-.297	3
409		15	max	242.96	1	6.764	9	3.065	3	0	0	15	.599	1
410		min	8.552	10	-73.427	3	-.127	2	0	1	-.002	1	-.281	3
411		16	max	299.636	2	172.822	2	3.041	3	0	0	3	.604	1
412		min	-102.661	3	-267.046	3	-.132	2	0	15	-.001	1	-.264	3
413		17	max	299.731	2	172.56	2	3.041	3	0	0	3	.609	1
414		min	-102.589	3	-267.243	3	-.132	2	0	15	0	1	-.206	3
415		18	max	-9.908	12	1486.95	1	2.912	1	0	.001	3	.293	1
416		min	-315.321	1	-509.839	3	-.019	10	0	1	0	2	-.096	3
417		19	max	-9.86	12	1486.688	1	2.912	1	0	.002	3	.014	3
418		min	-315.225	1	-510.036	3	-.019	10	0	1	0	2	-.029	1
419	M9	1	max	143.141	1	344.617	3	120.608	1	0	-.006	15	.014	1
420		min	4.778	15	-401.343	1	4.183	15	0	1	-.175	1	-.01	3
421		2	max	143.237	1	344.42	3	120.608	1	0	-.004	12	.101	1
422		min	4.807	15	-401.605	1	4.183	15	0	1	-.149	1	-.085	3
423		3	max	121.903	1	7.573	9	83.048	1	0	.001	3	.186	1
424		min	4.556	15	-21.378	3	1.612	12	0	15	-.121	1	-.158	3
425		4	max	121.998	1	7.355	9	83.048	1	0	.002	3	.186	1
426		min	4.585	15	-21.575	3	1.612	12	0	15	-.103	1	-.153	3
427		5	max	122.094	1	7.136	9	83.048	1	0	.002	3	.186	1
428		min	4.614	15	-21.772	3	1.612	12	0	15	-.085	1	-.148	3
429		6	max	122.189	1	6.917	9	83.048	1	0	.003	3	.186	1
430		min	4.642	15	-21.969	3	1.612	12	0	15	-.067	1	-.144	3
431		7	max	122.285	1	6.699	9	83.048	1	0	.003	3	.186	1
432		min	4.671	15	-22.165	3	1.612	12	0	15	-.049	1	-.139	3
433		8	max	122.38	1	6.48	9	83.048	1	0	.004	3	.187	1
434		min	4.7	15	-22.362	3	1.612	12	0	15	-.031	1	-.134	3
435		9	max	122.476	1	6.261	9	83.048	1	0	.004	3	.187	1
436		min	4.729	15	-22.559	3	1.612	12	0	15	-.013	1	-.129	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	122.571	1	6.043	9	83.048	1	0	1	.005	1	.187	1
438			min	4.758	15	-22.756	3	1.612	12	0	15	0	10	-.124	3
439		11	max	122.667	1	5.824	9	83.048	1	0	1	.023	1	.188	1
440			min	4.786	15	-22.953	3	1.612	12	0	15	0	15	-.119	3
441		12	max	122.762	1	5.605	9	83.048	1	0	1	.041	1	.188	1
442			min	4.815	15	-23.149	3	1.612	12	0	15	.001	15	-.114	3
443		13	max	122.858	1	5.387	9	83.048	1	0	1	.059	1	.189	1
444			min	4.844	15	-23.346	3	1.612	12	0	15	.002	15	-.109	3
445		14	max	122.953	1	5.168	9	83.048	1	0	1	.077	1	.189	1
446			min	4.873	15	-23.543	3	1.612	12	0	15	.003	15	-.104	3
447		15	max	123.049	1	4.949	9	83.048	1	0	1	.095	1	.19	1
448			min	4.902	15	-23.74	3	1.612	12	0	15	.003	15	-.099	3
449		16	max	83.18	2	27.656	10	83.939	1	0	15	.115	1	.191	1
450			min	-31.392	3	-88.882	3	1.638	12	0	1	.004	15	-.093	3
451		17	max	83.276	2	27.437	10	83.939	1	0	15	.133	1	.208	1
452			min	-31.32	3	-89.079	3	1.638	12	0	1	.004	15	-.074	3
453		18	max	-4.799	15	451.145	1	88.433	1	0	1	.152	1	.113	1
454			min	-143.018	1	-154.741	3	1.88	12	0	3	.005	15	-.041	3
455		19	max	-4.771	15	450.882	1	88.433	1	0	1	.171	1	.015	1
456			min	-142.923	1	-154.938	3	1.88	12	0	3	.006	15	-.007	3
457	M13	1	max	120.88	1	400.676	1	-4.778	15	.014	1	.175	1	0	1
458			min	4.183	15	-344.602	3	-143.123	1	-.01	3	.006	15	0	3
459		2	max	120.88	1	282.622	1	-3.665	15	.014	1	.055	1	.277	3
460			min	4.183	15	-242.999	3	-109.716	1	-.01	3	.002	15	-.323	1
461		3	max	120.88	1	164.567	1	-2.552	15	.014	1	.001	3	.459	3
462			min	4.183	15	-141.397	3	-76.309	1	-.01	3	-.032	1	-.534	1
463		4	max	120.88	1	46.513	1	-1.438	15	.014	1	-.002	12	.545	3
464			min	4.183	15	-39.795	3	-42.902	1	-.01	3	-.089	1	-.633	1
465		5	max	120.88	1	61.808	3	-.325	15	.014	1	-.003	12	.534	3
466			min	4.183	15	-71.542	1	-9.495	1	-.01	3	-.113	1	-.622	1
467		6	max	120.88	1	163.41	3	23.912	1	.014	1	-.003	12	.428	3
468			min	4.183	15	-189.596	1	.41	12	-.01	3	-.107	1	-.498	1
469		7	max	120.88	1	265.013	3	57.32	1	.014	1	-.002	12	.225	3
470			min	4.183	15	-307.651	1	1.496	12	-.01	3	-.068	1	-.264	1
471		8	max	120.88	1	366.615	3	90.727	1	.014	1	.002	1	.083	1
472			min	4.183	15	-425.705	1	2.582	12	-.01	3	0	3	-.073	3
473		9	max	120.88	1	468.218	3	124.134	1	.014	1	.103	1	.541	1
474			min	4.183	15	-543.76	1	3.668	12	-.01	3	.003	12	-.467	3
475		10	max	120.88	1	569.82	3	157.541	1	.011	2	.236	1	1.11	1
476			min	4.183	15	-661.814	1	4.754	12	-.014	1	.007	12	-.957	3
477		11	max	88.976	1	543.76	1	-3.57	12	.01	3	.098	1	.541	1
478			min	2.989	15	-468.218	3	-123.451	1	-.014	1	.001	12	-.467	3
479		12	max	88.976	1	425.705	1	-2.483	12	.01	3	0	10	.083	1
480			min	2.989	15	-366.615	3	-90.044	1	-.014	1	-.003	3	-.073	3
481		13	max	88.976	1	307.651	1	-1.397	12	.01	3	-.002	15	.225	3
482			min	2.989	15	-265.013	3	-56.636	1	-.014	1	-.072	1	-.264	1
483		14	max	88.976	1	189.596	1	-.311	12	.01	3	-.004	15	.428	3
484			min	2.989	15	-163.41	3	-23.229	1	-.014	1	-.11	1	-.498	1
485		15	max	88.976	1	71.542	1	10.178	1	.01	3	-.004	15	.534	3
486			min	2.989	15	-61.808	3	.349	15	-.014	1	-.116	1	-.622	1
487		16	max	88.976	1	39.795	3	43.585	1	.01	3	-.003	12	.545	3
488			min	2.989	15	-46.513	1	1.463	15	-.014	1	-.09	1	-.633	1
489		17	max	88.976	1	141.397	3	76.992	1	.01	3	0	12	.459	3
490			min	2.989	15	-164.567	1	2.576	15	-.014	1	-.033	1	-.534	1
491		18	max	88.976	1	243	3	110.399	1	.01	3	.055	1	.277	3
492			min	2.989	15	-282.622	1	3.689	15	-.014	1	.002	15	-.323	1
493		19	max	88.976	1	344.602	3	143.806	1	.01	3	.175	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	2.989	15	-400.676	1	4.803	15	-.014	1	.006	15	0	3
495		max	-1.88	12	451.592	1	-4.771	15	.007	3	.171	1	0	1
496		min	-88.094	1	-154.959	3	-142.937	1	-.015	1	.006	15	0	3
497		max	-1.88	12	318.526	1	-3.657	15	.007	3	.052	1	.125	3
498		min	-88.094	1	-109.399	3	-109.53	1	-.015	1	.002	15	-.364	1
499		max	-1.88	12	185.46	1	-2.544	15	.007	3	-.001	12	.207	3
500		min	-88.094	1	-63.839	3	-76.123	1	-.015	1	-.035	1	-.602	1
501		max	-1.88	12	52.394	1	-1.431	15	.007	3	-.003	15	.245	3
502		min	-88.094	1	-18.279	3	-42.716	1	-.015	1	-.092	1	-.714	1
503		max	-1.88	12	27.281	3	-.318	15	.007	3	-.004	15	.241	3
504	M15	min	-88.094	1	-80.672	1	-9.309	1	-.015	1	-.116	1	-.701	1
505		max	-1.88	12	72.841	3	24.098	1	.007	3	-.004	15	.194	3
506		min	-88.094	1	-213.738	1	.528	12	-.015	1	-.109	1	-.562	1
507		max	-1.88	12	118.401	3	57.505	1	.007	3	-.002	15	.104	3
508		min	-88.094	1	-346.804	1	1.614	12	-.015	1	-.071	1	-.297	1
509		max	-1.88	12	163.961	3	90.912	1	.007	3	.001	2	.094	1
510		min	-88.094	1	-479.87	1	2.7	12	-.015	1	-.002	3	-.03	3
511		max	-1.88	12	209.522	3	124.32	1	.007	3	.101	1	.61	1
512		min	-88.094	1	-612.936	1	3.786	12	-.015	1	.002	12	-.206	3
513		max	-3.061	15	-17.148	15	157.727	1	0	15	.235	1	1.251	1
514	M14	min	-91.033	1	-746.002	1	-7.461	3	-.015	1	.007	12	-.426	3
515		max	-3.061	15	612.936	1	-3.901	12	.015	1	.102	1	.61	1
516		min	-91.033	1	-209.522	3	-124.064	1	-.007	3	.003	12	-.206	3
517		max	-3.061	15	479.87	1	-2.815	12	.015	1	0	2	.094	1
518		min	-91.033	1	-163.961	3	-90.657	1	-.007	3	0	3	-.03	3
519		max	-3.061	15	346.804	1	-1.729	12	.015	1	-.002	12	.104	3
520		min	-91.033	1	-118.401	3	-57.249	1	-.007	3	-.07	1	-.297	1
521		max	-3.061	15	213.738	1	-.643	12	.015	1	-.003	12	.194	3
522		min	-91.033	1	-72.841	3	-23.842	1	-.007	3	-.108	1	-.562	1
523		max	-3.061	15	80.672	1	9.565	1	.015	1	-.004	12	.241	3
524	M13	min	-91.033	1	-27.281	3	.326	15	-.007	3	-.115	1	-.701	1
525		max	-3.061	15	18.279	3	42.972	1	.015	1	-.003	12	.245	3
526		min	-91.033	1	-52.394	1	1.439	15	-.007	3	-.09	1	-.714	1
527		max	-3.061	15	63.839	3	76.379	1	.015	1	0	12	.207	3
528		min	-91.033	1	-185.46	1	2.553	15	-.007	3	-.033	1	-.602	1
529		max	-3.061	15	109.399	3	109.786	1	.015	1	.054	1	.125	3
530		min	-91.033	1	-318.526	1	3.666	15	-.007	3	.002	15	-.364	1
531		max	-3.061	15	154.959	3	143.193	1	.015	1	.174	1	0	1
532		min	-91.033	1	-451.592	1	4.779	15	-.007	3	.006	15	0	3
533		max	0	10	2.958	4	.023	3	0	1	0	1	0	1
534	M12	min	-35.153	1	0	10	-.028	1	0	3	0	3	0	1
535		max	0	10	2.63	4	.023	3	0	1	0	1	0	10
536		min	-35.233	1	0	10	-.028	1	0	3	0	3	-.001	4
537		max	0	10	2.301	4	.023	3	0	1	0	1	0	10
538		min	-35.312	1	0	10	-.028	1	0	3	0	3	-.003	4
539		max	0	10	1.972	4	.023	3	0	1	0	1	0	10
540		min	-35.392	1	0	10	-.028	1	0	3	0	3	-.004	4
541		max	0	10	1.643	4	.023	3	0	1	0	1	0	10
542		min	-35.472	1	0	10	-.028	1	0	3	0	3	-.005	4
543		max	0	10	1.315	4	.023	3	0	1	0	1	0	10
544	M11	min	-35.551	1	0	10	-.028	1	0	3	0	3	-.005	4
545		max	0	10	.986	4	.023	3	0	1	0	3	0	10
546		min	-35.631	1	0	10	-.028	1	0	3	0	2	-.006	4
547		max	0	10	.657	4	.023	3	0	1	0	3	0	10
548		min	-35.71	1	0	10	-.028	1	0	3	0	1	-.006	4
549		max	0	10	.329	4	.023	3	0	1	0	3	0	10
550		min	-35.79	1	0	10	-.028	1	0	3	0	1	-.007	4



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	10	0	1	.023	3	0	1	0	3	0	10
552		min	-35.869	1	0	1	-.028	1	0	3	0	1	-.007	4
553	11	max	0	10	0	10	.023	3	0	1	0	3	0	10
554		min	-35.949	1	-.329	4	-.028	1	0	3	0	1	-.007	4
555	12	max	0	10	0	10	.023	3	0	1	0	3	0	10
556		min	-36.029	1	-.657	4	-.028	1	0	3	0	1	-.006	4
557	13	max	0	10	0	10	.023	3	0	1	0	3	0	10
558		min	-36.108	1	-.986	4	-.028	1	0	3	0	1	-.006	4
559	14	max	0	10	0	10	.023	3	0	1	0	3	0	10
560		min	-36.188	1	-1.315	4	-.028	1	0	3	0	1	-.005	4
561	15	max	0	10	0	10	.023	3	0	1	0	3	0	10
562		min	-36.267	1	-1.643	4	-.028	1	0	3	0	1	-.005	4
563	16	max	0	10	0	10	.023	3	0	1	0	3	0	10
564		min	-36.347	1	-1.972	4	-.028	1	0	3	0	1	-.004	4
565	17	max	0	10	0	10	.023	3	0	1	0	3	0	10
566		min	-36.426	1	-2.301	4	-.028	1	0	3	0	1	-.003	4
567	18	max	0	10	0	10	.023	3	0	1	0	3	0	10
568		min	-36.506	1	-2.63	4	-.028	1	0	3	0	1	-.001	4
569	19	max	0	10	0	10	.023	3	0	1	0	3	0	1
570		min	-36.585	1	-2.958	4	-.028	1	0	3	0	1	0	1
571	M16A	1	max	-.917	10	2.958	.02	1	0	3	0	3	0	1
572		min	-40.412	1	.695	15	-.009	3	0	1	0	1	0	1
573	2	max	-.851	10	2.63	4	.02	1	0	3	0	3	0	15
574		min	-40.333	1	.618	15	-.009	3	0	1	0	1	-.001	4
575	3	max	-.785	10	2.301	4	.02	1	0	3	0	3	0	15
576		min	-40.253	1	.541	15	-.009	3	0	1	0	1	-.003	4
577	4	max	-.718	10	1.972	4	.02	1	0	3	0	3	0	15
578		min	-40.174	1	.464	15	-.009	3	0	1	0	1	-.004	4
579	5	max	-.652	10	1.643	4	.02	1	0	3	0	3	-.001	15
580		min	-40.094	1	.386	15	-.009	3	0	1	0	1	-.005	4
581	6	max	-.586	10	1.315	4	.02	1	0	3	0	3	-.001	15
582		min	-40.015	1	.309	15	-.009	3	0	1	0	1	-.005	4
583	7	max	-.519	10	.986	4	.02	1	0	3	0	3	-.001	15
584		min	-39.935	1	.232	15	-.009	3	0	1	0	1	-.006	4
585	8	max	-.453	10	.657	4	.02	1	0	3	0	3	-.001	15
586		min	-39.855	1	.155	15	-.009	3	0	1	0	1	-.006	4
587	9	max	-.387	10	.329	4	.02	1	0	3	0	3	-.002	15
588		min	-39.776	1	.077	15	-.009	3	0	1	0	1	-.007	4
589	10	max	-.321	10	0	1	.02	1	0	3	0	3	-.002	15
590		min	-39.696	1	0	1	-.009	3	0	1	0	1	-.007	4
591	11	max	-.254	10	-.077	15	.02	1	0	3	0	3	-.002	15
592		min	-39.617	1	-.329	4	-.009	3	0	1	0	1	-.007	4
593	12	max	-.188	10	-.155	15	.02	1	0	3	0	3	-.001	15
594		min	-39.537	1	-.657	4	-.009	3	0	1	0	1	-.006	4
595	13	max	-.122	10	-.232	15	.02	1	0	3	0	1	-.001	15
596		min	-39.458	1	-.986	4	-.009	3	0	1	0	13	-.006	4
597	14	max	-.055	10	-.309	15	.02	1	0	3	0	1	-.001	15
598		min	-39.378	1	-1.315	4	-.009	3	0	1	0	3	-.005	4
599	15	max	.011	10	-.386	15	.02	1	0	3	0	1	-.001	15
600		min	-39.299	1	-1.643	4	-.009	3	0	1	0	3	-.005	4
601	16	max	.077	10	-.464	15	.02	1	0	3	0	1	0	15
602		min	-39.219	1	-1.972	4	-.009	3	0	1	0	3	-.004	4
603	17	max	.144	10	-.541	15	.02	1	0	3	0	1	0	15
604		min	-39.139	1	-2.301	4	-.009	3	0	1	0	3	-.003	4
605	18	max	.21	10	-.618	15	.02	1	0	3	0	1	0	15
606		min	-39.06	1	-2.63	4	-.009	3	0	1	0	3	-.001	4
607	19	max	.276	10	-.695	15	.02	1	0	3	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-38.98	1	-2.958	4	-.009	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.008	2	.016	1	-4.506e-5	15	NC	3	NC	3	
2			min	-.003	3	-.006	3	0	3	-1.345e-3	1	4388.4	2	2023.301	1	
3			2	max	.003	1	.007	2	.015	1	-4.32e-5	15	NC	3	NC	3
4				min	-.003	3	-.006	3	0	3	-1.29e-3	1	4756.155	2	2189.494	1
5			3	max	.003	1	.006	2	.014	1	-4.133e-5	15	NC	3	NC	3
6				min	-.003	3	-.006	3	0	3	-1.235e-3	1	5187.869	2	2385.177	1
7			4	max	.003	1	.006	2	.013	1	-3.947e-5	15	NC	3	NC	3
8				min	-.002	3	-.006	3	0	3	-1.18e-3	1	5698.011	2	2617.5	1
9			5	max	.003	1	.005	2	.011	1	-3.76e-5	15	NC	3	NC	3
10				min	-.002	3	-.005	3	0	3	-1.124e-3	1	6305.599	2	2896.003	1
11		6	max	.002	1	.005	2	.01	1	-3.573e-5	15	NC	1	NC	3	
12			min	-.002	3	-.005	3	0	3	-1.069e-3	1	7036.028	2	3233.647	1	
13		7	max	.002	1	.004	2	.009	1	-3.387e-5	15	NC	1	NC	3	
14			min	-.002	3	-.005	3	0	3	-1.014e-3	1	7923.844	2	3648.412	1	
15		8	max	.002	1	.004	2	.008	1	-3.2e-5	15	NC	1	NC	2	
16			min	-.002	3	-.005	3	0	3	-9.583e-4	1	9017.072	2	4165.843	1	
17		9	max	.002	1	.003	2	.007	1	-3.013e-5	15	NC	1	NC	2	
18			min	-.002	3	-.004	3	0	3	-9.03e-4	1	NC	1	4823.261	1	
19		10	max	.002	1	.003	2	.006	1	-2.827e-5	15	NC	1	NC	2	
20			min	-.001	3	-.004	3	0	3	-8.478e-4	1	NC	1	5677.022	1	
21		11	max	.001	1	.002	2	.005	1	-2.64e-5	15	NC	1	NC	2	
22			min	-.001	3	-.004	3	0	3	-7.925e-4	1	NC	1	6815.648	1	
23		12	max	.001	1	.002	2	.004	1	-2.454e-5	15	NC	1	NC	2	
24			min	-.001	3	-.003	3	0	3	-7.372e-4	1	NC	1	8385.075	1	
25		13	max	.001	1	.002	2	.003	1	-2.267e-5	15	NC	1	NC	1	
26			min	0	3	-.003	3	0	3	-6.819e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-2.08e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-6.266e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.002	1	-1.894e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-5.713e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	.001	1	-1.707e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-5.16e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.52e-5	15	NC	1	NC	1	
34			min	0	3	-.001	3	0	12	-4.607e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.334e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-4.054e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-9.843e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.501e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.609e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	4.627e-6	12	NC	1	NC	1	
41			2	max	0	9	0	2	0	12	2.032e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	6.705e-6	15	NC	1	NC	1
43			3	max	0	9	0	2	0	12	2.455e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	-.001	1	8.134e-6	15	NC	1	NC	1
45			4	max	0	9	0	2	0	12	2.878e-4	1	NC	1	NC	1
46				min	0	2	-.002	3	-.001	1	9.564e-6	15	NC	1	NC	1
47			5	max	0	9	0	2	0	12	3.301e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	-.001	1	1.099e-5	15	NC	1	NC	1
49			6	max	0	9	0	2	0	3	3.725e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.242e-5	15	NC	1	NC	1
51		7	max	0	9	.001	2	0	3	4.148e-4	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.004	3	-.001	1	1.385e-5	15	NC	1	NC	1
53	8	max	0	9	.001	2	0	3	4.571e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	1.528e-5	15	NC	1	NC	1
55	9	max	0	9	.002	2	0	3	4.994e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	2	1.671e-5	15	NC	1	NC	1
57	10	max	0	9	.002	2	0	1	5.417e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	15	1.814e-5	15	NC	1	NC	1
59	11	max	0	9	.003	2	.001	1	5.84e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	15	1.957e-5	15	NC	1	NC	1
61	12	max	0	9	.003	2	.002	1	6.263e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	2.1e-5	15	NC	1	NC	1
63	13	max	0	9	.004	2	.003	1	6.686e-4	1	NC	1	NC	1
64		min	0	2	-.007	3	0	15	2.243e-5	15	NC	1	NC	1
65	14	max	0	9	.005	2	.004	1	7.109e-4	1	NC	1	NC	1
66		min	0	2	-.007	3	0	15	2.386e-5	15	9439.052	2	NC	1
67	15	max	0	9	.006	2	.005	1	7.532e-4	1	NC	3	NC	2
68		min	0	2	-.007	3	0	15	2.529e-5	15	7983.097	2	9015.947	1
69	16	max	.001	9	.007	2	.006	1	7.955e-4	1	NC	3	NC	2
70		min	0	2	-.008	3	0	15	2.672e-5	15	6845.188	2	7582.102	1
71	17	max	.001	9	.008	2	.007	1	8.378e-4	1	NC	3	NC	2
72		min	0	2	-.008	3	0	15	2.815e-5	15	5947.645	2	6558.176	1
73	18	max	.001	9	.009	2	.008	1	8.801e-4	1	NC	3	NC	2
74		min	0	2	-.008	3	0	15	2.958e-5	15	5234.021	2	5804.097	1
75	19	max	.001	9	.01	2	.009	1	9.224e-4	1	NC	3	NC	2
76		min	0	2	-.008	3	0	15	3.101e-5	15	4663.081	2	5237.703	1
77	M4	1	max	.003	1	.009	2	15	-3.704e-5	15	NC	1	NC	3
78		min	0	3	-.007	3	-.007	1	-1.115e-3	1	NC	1	2928.195	1
79	2	max	.003	1	.008	2	0	15	-3.704e-5	15	NC	1	NC	3
80		min	0	3	-.006	3	-.006	1	-1.115e-3	1	NC	1	3194.048	1
81	3	max	.002	1	.008	2	0	15	-3.704e-5	15	NC	1	NC	3
82		min	0	3	-.006	3	-.006	1	-1.115e-3	1	NC	1	3510.471	1
83	4	max	.002	1	.007	2	0	15	-3.704e-5	15	NC	1	NC	2
84		min	0	3	-.006	3	-.005	1	-1.115e-3	1	NC	1	3890.802	1
85	5	max	.002	1	.007	2	0	15	-3.704e-5	15	NC	1	NC	2
86		min	0	3	-.005	3	-.004	1	-1.115e-3	1	NC	1	4353.206	1
87	6	max	.002	1	.006	2	0	15	-3.704e-5	15	NC	1	NC	2
88		min	0	3	-.005	3	-.004	1	-1.115e-3	1	NC	1	4922.953	1
89	7	max	.002	1	.006	2	0	15	-3.704e-5	15	NC	1	NC	2
90		min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	5636.037	1
91	8	max	.002	1	.005	2	0	15	-3.704e-5	15	NC	1	NC	2
92		min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	6545.167	1
93	9	max	.002	1	.005	2	0	15	-3.704e-5	15	NC	1	NC	2
94		min	0	3	-.004	3	-.003	1	-1.115e-3	1	NC	1	7730.077	1
95	10	max	.001	1	.004	2	0	15	-3.704e-5	15	NC	1	NC	2
96		min	0	3	-.003	3	-.002	1	-1.115e-3	1	NC	1	9316.126	1
97	11	max	.001	1	.004	2	0	15	-3.704e-5	15	NC	1	NC	1
98		min	0	3	-.003	3	-.002	1	-1.115e-3	1	NC	1	NC	1
99	12	max	.001	1	.003	2	0	15	-3.704e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	-.001	1	-1.115e-3	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-3.704e-5	15	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-1.115e-3	1	NC	1	NC	1
103	14	max	0	1	.002	2	0	15	-3.704e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-1.115e-3	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-3.704e-5	15	NC	1	NC	1
106		min	0	3	-.001	3	0	1	-1.115e-3	1	NC	1	NC	1
107	16	max	0	1	.001	2	0	15	-3.704e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-1.115e-3	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-3.704e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-1.115e-3	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.704e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-1.115e-3	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.704e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.115e-3	1	NC	1	NC	1
115	M6	1	max	.011	1	.025	2	.005	1	2.761e-4	1	NC	3	NC	2
116			min	-.01	3	-.019	3	-.002	3	4.542e-6	10	1338.034	2	7233.353	1
117		2	max	.01	1	.023	2	.004	1	2.591e-4	1	NC	3	NC	2
118			min	-.009	3	-.018	3	-.002	3	3.598e-6	10	1428.134	2	7859.774	1
119		3	max	.01	1	.022	2	.004	1	2.422e-4	1	NC	3	NC	2
120			min	-.009	3	-.017	3	-.002	3	2.653e-6	10	1530.926	2	8603.764	1
121		4	max	.009	1	.02	2	.004	1	2.252e-4	1	NC	3	NC	2
122			min	-.008	3	-.016	3	-.002	3	1.709e-6	10	1648.945	2	9495.481	1
123		5	max	.008	1	.019	2	.003	1	2.083e-4	1	NC	3	NC	1
124			min	-.008	3	-.015	3	-.002	3	7.646e-7	10	1785.454	2	NC	1
125		6	max	.008	1	.017	2	.003	1	1.913e-4	1	NC	3	NC	1
126			min	-.007	3	-.014	3	-.001	3	-1.797e-7	10	1944.724	2	NC	1
127		7	max	.007	1	.016	2	.002	1	1.744e-4	1	NC	3	NC	1
128			min	-.007	3	-.013	3	-.001	3	-1.124e-6	10	2132.454	2	NC	1
129		8	max	.007	1	.014	2	.002	1	1.574e-4	1	NC	3	NC	1
130			min	-.006	3	-.012	3	-.001	3	-2.068e-6	10	2356.417	2	NC	1
131		9	max	.006	1	.013	2	.002	1	1.43e-4	3	NC	3	NC	1
132			min	-.005	3	-.011	3	-.001	3	-3.013e-6	10	2627.496	2	NC	1
133		10	max	.005	1	.011	2	.002	1	1.379e-4	3	NC	3	NC	1
134			min	-.005	3	-.01	3	0	3	-3.957e-6	10	2961.418	2	NC	1
135		11	max	.005	1	.01	2	.001	1	1.328e-4	3	NC	3	NC	1
136			min	-.004	3	-.009	3	0	3	-4.901e-6	10	3381.769	2	NC	1
137		12	max	.004	1	.008	2	0	1	1.277e-4	3	NC	3	NC	1
138			min	-.004	3	-.008	3	0	3	-8.948e-6	2	3925.614	2	NC	1
139		13	max	.004	1	.007	2	0	1	1.227e-4	3	NC	3	NC	1
140			min	-.003	3	-.007	3	0	3	-1.409e-5	2	4654.724	2	NC	1
141		14	max	.003	1	.006	2	0	1	1.176e-4	3	NC	3	NC	1
142			min	-.003	3	-.006	3	0	3	-1.924e-5	2	5680.278	2	NC	1
143		15	max	.002	1	.005	2	0	1	1.125e-4	3	NC	3	NC	1
144			min	-.002	3	-.005	3	0	3	-2.439e-5	2	7224.627	2	NC	1
145		16	max	.002	1	.003	2	0	1	1.074e-4	3	NC	1	NC	1
146			min	-.002	3	-.003	3	0	3	-2.953e-5	2	9806.561	2	NC	1
147		17	max	.001	1	.002	2	0	1	1.023e-4	3	NC	1	NC	1
148			min	-.001	3	-.002	3	0	3	-3.468e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	9.726e-5	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-3.982e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	9.218e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.497e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.037e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-4.218e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.73e-5	2	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-3.162e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.536e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-2.107e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.667e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	2	-1.051e-5	3	NC	1	NC	1
161		5	max	0	3	.006	2	0	3	1.798e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	2	4.338e-8	3	8301.046	2	NC	1
163		6	max	0	3	.007	2	0	3	1.929e-5	1	NC	3	NC	1
164			min	-.001	2	-.008	3	0	2	5.69e-7	15	6651.519	2	NC	1
165		7	max	.001	3	.008	2	.001	3	2.115e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.001	2	-.009	3	0	2	6.692e-7	15	5524.683	2	NC	1
167		8	max	.001	3	.01	2	.001	3	3.171e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-1.126e-6	2	4699.497	2	NC	1
169		9	max	.001	3	.011	2	.001	3	4.227e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-4.197e-6	2	4065.794	2	NC	1
171		10	max	.002	3	.013	2	.001	3	5.282e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	1	-7.268e-6	2	3562.497	2	NC	1
173		11	max	.002	3	.015	2	.001	3	6.338e-5	3	NC	3	NC	1
174			min	-.002	2	-.015	3	-.001	1	-1.034e-5	2	3152.928	2	NC	1
175		12	max	.002	3	.016	2	.001	3	7.393e-5	3	NC	3	NC	1
176			min	-.002	2	-.016	3	-.001	1	-1.341e-5	2	2813.62	2	NC	1
177		13	max	.002	3	.018	2	.001	3	8.449e-5	3	NC	3	NC	1
178			min	-.002	2	-.017	3	-.001	1	-1.648e-5	2	2528.757	2	NC	1
179		14	max	.002	3	.02	2	.001	3	9.504e-5	3	NC	3	NC	1
180			min	-.003	2	-.018	3	-.002	1	-1.955e-5	2	2287.198	2	NC	1
181		15	max	.002	3	.022	2	.001	3	1.056e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	-.002	1	-2.262e-5	2	2080.808	2	NC	1
183		16	max	.003	3	.024	2	.001	3	1.162e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	-.002	1	-2.57e-5	2	1903.459	2	NC	1
185		17	max	.003	3	.026	2	.001	3	1.267e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	-.002	1	-2.877e-5	2	1750.425	2	NC	1
187		18	max	.003	3	.028	2	.001	3	1.373e-4	3	NC	3	NC	1
188			min	-.004	2	-.021	3	-.002	1	-3.184e-5	2	1617.984	2	NC	1
189		19	max	.003	3	.031	2	.001	3	1.478e-4	3	NC	3	NC	1
190			min	-.004	2	-.022	3	-.002	1	-3.491e-5	2	1503.167	2	NC	1
191	M8	1	max	.008	1	.028	2	.003	1	-1.969e-6	10	NC	1	NC	2
192			min	-.001	3	-.019	3	0	3	-1.609e-4	1	NC	1	7322.42	1
193		2	max	.007	1	.027	2	.002	1	-1.969e-6	10	NC	1	NC	2
194			min	-.001	3	-.018	3	0	3	-1.609e-4	1	NC	1	7983.416	1
195		3	max	.007	1	.025	2	.002	1	-1.969e-6	10	NC	1	NC	2
196			min	-.001	3	-.017	3	0	3	-1.609e-4	1	NC	1	8770.333	1
197		4	max	.006	1	.024	2	.002	1	-1.969e-6	10	NC	1	NC	2
198			min	-.001	3	-.016	3	0	3	-1.609e-4	1	NC	1	9716.34	1
199		5	max	.006	1	.022	2	.002	1	-1.969e-6	10	NC	1	NC	1
200			min	0	3	-.015	3	0	3	-1.609e-4	1	NC	1	NC	1
201		6	max	.006	1	.021	2	.002	1	-1.969e-6	10	NC	1	NC	1
202			min	0	3	-.014	3	0	3	-1.609e-4	1	NC	1	NC	1
203		7	max	.005	1	.019	2	.001	1	-1.969e-6	10	NC	1	NC	1
204			min	0	3	-.013	3	0	3	-1.609e-4	1	NC	1	NC	1
205		8	max	.005	1	.017	2	.001	1	-1.969e-6	10	NC	1	NC	1
206			min	0	3	-.012	3	0	3	-1.609e-4	1	NC	1	NC	1
207		9	max	.004	1	.016	2	.001	1	-1.969e-6	10	NC	1	NC	1
208			min	0	3	-.011	3	0	3	-1.609e-4	1	NC	1	NC	1
209		10	max	.004	1	.014	2	0	1	-1.969e-6	10	NC	1	NC	1
210			min	0	3	-.01	3	0	3	-1.609e-4	1	NC	1	NC	1
211		11	max	.003	1	.013	2	0	1	-1.969e-6	10	NC	1	NC	1
212			min	0	3	-.009	3	0	3	-1.609e-4	1	NC	1	NC	1
213		12	max	.003	1	.011	2	0	1	-1.969e-6	10	NC	1	NC	1
214			min	0	3	-.008	3	0	3	-1.609e-4	1	NC	1	NC	1
215		13	max	.003	1	.009	2	0	1	-1.969e-6	10	NC	1	NC	1
216			min	0	3	-.006	3	0	3	-1.609e-4	1	NC	1	NC	1
217		14	max	.002	1	.008	2	0	1	-1.969e-6	10	NC	1	NC	1
218			min	0	3	-.005	3	0	3	-1.609e-4	1	NC	1	NC	1
219		15	max	.002	1	.006	2	0	1	-1.969e-6	10	NC	1	NC	1
220			min	0	3	-.004	3	0	3	-1.609e-4	1	NC	1	NC	1
221		16	max	.001	1	.005	2	0	1	-1.969e-6	10	NC	1	NC	1
222			min	0	3	-.003	3	0	3	-1.609e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-1.969e-6	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.609e-4	1	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.969e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.609e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.969e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.609e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.008	2	0	3	1.119e-3	1	NC	3	NC	1
230			min	-.003	3	-.006	3	-.002	1	-1.716e-4	3	4393.084	2	NC	1
231		2	max	.003	1	.007	2	0	3	1.06e-3	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.002	1	-1.671e-4	3	4749.389	2	NC	1
233		3	max	.003	1	.006	2	0	3	1.001e-3	1	NC	3	NC	1
234			min	-.003	3	-.006	3	-.002	1	-1.625e-4	3	5165.569	2	NC	1
235		4	max	.003	1	.006	2	0	3	9.427e-4	1	NC	3	NC	1
236			min	-.002	3	-.006	3	-.002	1	-1.58e-4	3	5654.671	2	NC	1
237		5	max	.003	1	.005	2	0	3	8.84e-4	1	NC	3	NC	1
238			min	-.002	3	-.006	3	-.002	1	-1.535e-4	3	6233.721	2	NC	1
239		6	max	.002	1	.005	2	0	3	8.252e-4	1	NC	1	NC	1
240			min	-.002	3	-.005	3	-.002	1	-1.489e-4	3	6925.281	2	NC	1
241		7	max	.002	1	.004	2	0	3	7.665e-4	1	NC	1	NC	1
242			min	-.002	3	-.005	3	-.001	1	-1.444e-4	3	7759.802	2	NC	1
243		8	max	.002	1	.004	2	0	3	7.077e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	-.001	1	-1.398e-4	3	8779.264	2	NC	1
245		9	max	.002	1	.003	2	0	3	6.49e-4	1	NC	1	NC	1
246			min	-.002	3	-.004	3	-.001	1	-1.353e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	5.902e-4	1	NC	1	NC	1
248			min	-.001	3	-.004	3	0	1	-1.307e-4	3	NC	1	NC	1
249		11	max	.002	1	.002	2	0	3	5.315e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.262e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.727e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.216e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	4.14e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-1.171e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.552e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.125e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.965e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.08e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.378e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.034e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.79e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-9.889e-5	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.203e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-9.434e-5	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.151e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.979e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	4.129e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-2.991e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	2	2.835e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.065e-4	1	NC	1	NC	1
271		3	max	0	9	0	2	0	2	1.54e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.83e-4	1	NC	1	NC	1
273		4	max	0	9	0	2	0	10	2.458e-6	3	NC	1	NC	1
274			min	0	2	-.002	3	0	1	-2.596e-4	1	NC	1	NC	1
275		5	max	0	9	0	2	0	10	-7.409e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	-.001	1	-3.362e-4	1	NC	1	NC	1
277		6	max	0	9	0	2	0	15	-1.38e-5	15	NC	1	NC	1
278			min	0	2	-.004	3	-.002	1	-4.127e-4	1	NC	1	NC	1
279		7	max	0	9	.001	2	0	15	-1.641e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.003	1	-4.893e-4	1	NC	1	NC	1
281		8	max	0	9	.001	2	0	15	-1.902e-5	15	NC	1	NC	1
282			min	0	2	-.005	3	-.004	1	-5.659e-4	1	NC	1	NC	1
283		9	max	0	9	.002	2	0	15	-2.163e-5	15	NC	1	NC	2
284			min	0	2	-.006	3	-.005	1	-6.424e-4	1	NC	1	9057.405	1
285		10	max	0	9	.002	2	0	15	-2.424e-5	15	NC	1	NC	2
286			min	0	2	-.006	3	-.006	1	-7.19e-4	1	NC	1	7301.318	1
287		11	max	0	9	.003	2	0	15	-2.685e-5	15	NC	1	NC	2
288			min	0	2	-.007	3	-.008	1	-7.956e-4	1	NC	1	6067.418	1
289		12	max	0	9	.003	2	0	15	-2.946e-5	15	NC	1	NC	2
290			min	0	2	-.007	3	-.009	1	-8.721e-4	1	NC	1	5167.805	1
291		13	max	0	9	.004	2	0	15	-3.207e-5	15	NC	1	NC	2
292			min	0	2	-.007	3	-.01	1	-9.487e-4	1	NC	1	4492.889	1
293		14	max	0	9	.005	2	0	15	-3.468e-5	15	NC	1	NC	2
294			min	0	2	-.007	3	-.012	1	-1.025e-3	1	9237.765	2	3975.219	1
295		15	max	0	9	.006	2	0	15	-3.729e-5	15	NC	3	NC	2
296			min	0	2	-.008	3	-.013	1	-1.102e-3	1	7865.79	2	3571.444	1
297		16	max	.001	9	.007	2	0	15	-3.99e-5	15	NC	3	NC	3
298			min	0	2	-.008	3	-.014	1	-1.178e-3	1	6780.736	2	3252.729	1
299		17	max	.001	9	.008	2	0	15	-4.251e-5	15	NC	3	NC	3
300			min	0	2	-.008	3	-.015	1	-1.255e-3	1	5916.691	2	2999.365	1
301		18	max	.001	9	.009	2	0	15	-4.512e-5	15	NC	3	NC	3
302			min	0	2	-.008	3	-.016	1	-1.332e-3	1	5224.422	2	2797.615	1
303		19	max	.001	9	.01	2	0	15	-4.773e-5	15	NC	3	NC	3
304			min	0	2	-.008	3	-.017	1	-1.408e-3	1	4667.158	2	2637.801	1
305	M12	1	max	.003	1	.009	2	.014	1	1.234e-3	1	NC	1	NC	3
306			min	0	3	-.007	3	0	15	4.238e-5	15	NC	1	1343.778	1
307		2	max	.003	1	.008	2	.013	1	1.234e-3	1	NC	1	NC	3
308			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1465.416	1
309		3	max	.002	1	.008	2	.012	1	1.234e-3	1	NC	1	NC	3
310			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1610.21	1
311		4	max	.002	1	.007	2	.011	1	1.234e-3	1	NC	1	NC	3
312			min	0	3	-.006	3	0	15	4.238e-5	15	NC	1	1784.263	1
313		5	max	.002	1	.007	2	.01	1	1.234e-3	1	NC	1	NC	3
314			min	0	3	-.005	3	0	15	4.238e-5	15	NC	1	1995.888	1
315		6	max	.002	1	.006	2	.009	1	1.234e-3	1	NC	1	NC	3
316			min	0	3	-.005	3	0	15	4.238e-5	15	NC	1	2256.65	1
317		7	max	.002	1	.006	2	.007	1	1.234e-3	1	NC	1	NC	3
318			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	2583.019	1
319		8	max	.002	1	.005	2	.006	1	1.234e-3	1	NC	1	NC	3
320			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	2999.118	1
321		9	max	.002	1	.005	2	.005	1	1.234e-3	1	NC	1	NC	3
322			min	0	3	-.004	3	0	15	4.238e-5	15	NC	1	3541.432	1
323		10	max	.001	1	.004	2	.005	1	1.234e-3	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	4267.324	1
325		11	max	.001	1	.004	2	.004	1	1.234e-3	1	NC	1	NC	2
326			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	5271.279	1
327		12	max	.001	1	.003	2	.003	1	1.234e-3	1	NC	1	NC	2
328			min	0	3	-.003	3	0	15	4.238e-5	15	NC	1	6718.128	1
329		13	max	0	1	.003	2	.002	1	1.234e-3	1	NC	1	NC	2
330			min	0	3	-.002	3	0	15	4.238e-5	15	NC	1	8917.086	1
331		14	max	0	1	.002	2	.002	1	1.234e-3	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	4.238e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	.001	1	1.234e-3	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	4.238e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	1.234e-3	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	4.238e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	1.234e-3	1	NC	1	NC	1
338			min	0	3	0	3	0	15	4.238e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	1.234e-3	1	NC	1	NC	1
340			min	0	3	0	3	0	15	4.238e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	1.234e-3	1	NC	1	NC	1
342			min	0	1	0	1	0	1	4.238e-5	15	NC	1	NC	1
343	M1	1	max	.006	3	.023	3	.001	3	1.996e-2	1	NC	1	NC	1
344			min	-.007	2	-.03	1	-.005	1	-1.708e-2	3	NC	1	NC	1
345		2	max	.006	3	.013	3	0	3	9.51e-3	1	NC	4	NC	2
346			min	-.007	2	-.016	1	-.012	1	-8.456e-3	3	3340.165	1	6822.313	1
347		3	max	.006	3	.003	3	0	3	5.896e-6	3	NC	5	NC	2
348			min	-.007	2	-.003	1	-.016	1	-7.453e-4	1	1727.586	1	4135.471	1
349		4	max	.006	3	.008	1	0	3	1.033e-5	3	NC	5	NC	3
350			min	-.007	2	-.005	3	-.019	1	-6.206e-4	1	1222.404	1	3420.212	1
351		5	max	.006	3	.017	1	0	3	1.476e-5	3	NC	5	NC	3
352			min	-.007	2	-.012	3	-.019	1	-4.959e-4	1	979.677	1	3281.65	1
353		6	max	.006	3	.025	1	0	12	1.919e-5	3	NC	5	NC	3
354			min	-.007	2	-.017	3	-.018	1	-3.712e-4	1	842.57	1	3507.251	1
355		7	max	.006	3	.031	1	0	12	2.362e-5	3	NC	5	NC	2
356			min	-.007	2	-.021	3	-.016	1	-2.464e-4	1	759.651	1	4167.804	1
357		8	max	.006	3	.035	1	0	3	2.805e-5	3	NC	5	NC	2
358			min	-.007	2	-.023	3	-.013	1	-1.217e-4	1	709.539	1	5699.104	1
359		9	max	.006	3	.037	1	0	3	3.248e-5	3	NC	5	NC	1
360			min	-.008	2	-.025	3	-.009	1	-9.087e-6	2	682.257	1	NC	1
361		10	max	.006	3	.038	1	0	3	1.278e-4	1	NC	5	NC	1
362			min	-.008	2	-.025	3	-.005	1	4.568e-6	15	673.211	1	NC	1
363		11	max	.006	3	.037	1	0	3	2.525e-4	1	NC	5	NC	1
364			min	-.008	2	-.024	3	-.001	1	8.732e-6	15	681.018	1	NC	1
365		12	max	.006	3	.035	1	.002	1	3.772e-4	1	NC	5	NC	2
366			min	-.008	2	-.022	3	0	15	1.29e-5	15	706.929	1	6710.113	1
367		13	max	.006	3	.03	1	.006	1	5.019e-4	1	NC	5	NC	2
368			min	-.008	2	-.019	3	0	15	1.706e-5	15	755.36	1	4645.434	1
369		14	max	.006	3	.024	1	.008	1	6.267e-4	1	NC	5	NC	3
370			min	-.008	2	-.015	3	0	15	2.122e-5	15	835.976	1	3803.302	1
371		15	max	.006	3	.016	1	.009	1	7.514e-4	1	NC	5	NC	3
372			min	-.008	2	-.01	3	0	15	2.539e-5	15	969.491	1	3500.014	1
373		16	max	.006	3	.007	1	.009	1	8.416e-4	1	NC	5	NC	3
374			min	-.008	2	-.004	3	0	15	2.842e-5	15	1205.294	1	3605.085	1
375		17	max	.006	3	.002	3	.007	1	1.116e-4	1	NC	4	NC	2
376			min	-.008	2	-.005	1	0	15	4.551e-6	15	1691.764	1	4323.391	1
377		18	max	.006	3	.01	3	.002	1	1.119e-2	1	NC	4	NC	2
378			min	-.008	2	-.018	1	0	15	-3.871e-3	3	3260.856	1	7091.607	1
379		19	max	.006	3	.018	3	0	3	2.248e-2	1	NC	1	NC	1
380			min	-.008	2	-.033	1	-.004	1	-7.844e-3	3	NC	1	NC	1
381	M5	1	max	.018	3	.069	3	.001	3	6.335e-7	1	NC	1	NC	1
382			min	-.024	2	-.092	1	-.006	1	4.46e-8	15	NC	1	NC	1
383		2	max	.018	3	.039	3	.002	3	4.629e-5	3	NC	5	NC	1
384			min	-.024	2	-.05	1	-.005	1	-9.154e-5	1	1115.297	1	NC	1
385		3	max	.018	3	.01	3	.002	3	9.118e-5	3	NC	5	NC	1
386			min	-.024	2	-.011	1	-.005	1	-1.818e-4	1	574.191	1	NC	1
387		4	max	.018	3	.022	1	.003	3	8.994e-5	3	NC	5	NC	1
388			min	-.024	2	-.013	3	-.004	1	-1.697e-4	1	405.219	1	NC	1
389		5	max	.018	3	.051	1	.003	3	8.87e-5	3	NC	15	NC	1
390			min	-.024	2	-.033	3	-.003	1	-1.575e-4	1	323.983	1	NC	1
391		6	max	.018	3	.074	1	.003	3	8.746e-5	3	NC	15	NC	1
392			min	-.024	2	-.048	3	-.003	1	-1.454e-4	1	278.014	1	NC	1
393		7	max	.018	3	.092	1	.003	3	8.623e-5	3	NC	15	NC	1

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-0.024	2	-0.06	3	-0.003	1	-1.333e-4	1	250.117	1	NC	1
395		8	max	.018	3	.105	1	.003	3	8.499e-5	3	NC	15	NC	1
396			min	-0.024	2	-0.067	3	-0.002	1	-1.211e-4	1	233.139	1	NC	1
397		9	max	.018	3	.113	1	.003	3	8.375e-5	3	9737.909	15	NC	1
398			min	-.025	2	-.072	3	-.002	1	-1.09e-4	1	223.739	1	NC	1
399		10	max	.018	3	.116	1	.003	3	8.251e-5	3	9637.71	15	NC	1
400			min	-.025	2	-.072	3	-.002	1	-9.689e-5	1	220.368	1	NC	1
401		11	max	.018	3	.113	1	.003	3	8.128e-5	3	9777.439	15	NC	1
402			min	-.025	2	-.07	3	-.002	1	-8.476e-5	1	222.544	1	NC	1
403		12	max	.018	3	.106	1	.002	3	8.004e-5	3	NC	15	NC	1
404			min	-.025	2	-.064	3	-.002	1	-7.264e-5	1	230.658	1	NC	1
405		13	max	.018	3	.092	1	.002	3	7.88e-5	3	NC	15	NC	1
406			min	-.025	2	-.055	3	-.002	1	-6.051e-5	1	246.139	1	NC	1
407		14	max	.018	3	.074	1	.002	3	7.756e-5	3	NC	15	NC	1
408			min	-.025	2	-.043	3	-.002	1	-4.838e-5	1	272.146	1	NC	1
409		15	max	.018	3	.049	1	.001	3	7.633e-5	3	NC	15	NC	1
410			min	-.025	2	-.029	3	-.002	1	-3.903e-5	2	315.484	1	NC	1
411		16	max	.018	3	.02	1	.001	3	7.288e-5	3	NC	5	NC	1
412			min	-.025	2	-.012	3	-.002	1	-3.664e-5	2	392.51	1	NC	1
413		17	max	.018	3	.007	3	0	3	1.69e-5	3	NC	5	NC	1
414			min	-.025	2	-.015	1	-.003	1	-2.418e-4	1	553.417	1	NC	1
415		18	max	.018	3	.029	3	0	3	7.954e-6	3	NC	5	NC	1
416			min	-.025	2	-.057	1	-.003	1	-1.24e-4	1	1072.459	1	NC	1
417		19	max	.018	3	.051	3	0	3	0	15	NC	1	NC	1
418			min	-.025	2	-.101	1	-.003	1	-1.103e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.023	3	0	3	1.708e-2	3	NC	1	NC	1
420			min	-.007	2	-.03	1	-.008	1	-1.996e-2	1	NC	1	NC	1
421		2	max	.006	3	.013	3	0	3	8.464e-3	3	NC	4	NC	2
422			min	-.007	2	-.016	1	-.001	1	-9.817e-3	1	3340.948	1	8112.281	1
423		3	max	.006	3	.003	3	.002	1	1.364e-4	1	NC	5	NC	2
424			min	-.007	2	-.003	1	0	3	4.741e-6	15	1728.001	1	5060.959	1
425		4	max	.006	3	.008	1	.004	1	3.029e-5	1	NC	5	NC	3
426			min	-.007	2	-.005	3	0	3	-1.712e-7	3	1222.694	1	4309.198	1
427		5	max	.006	3	.017	1	.005	1	-6.036e-7	10	NC	5	NC	3
428			min	-.007	2	-.012	3	0	3	-7.583e-5	1	979.897	1	4299.353	1
429		6	max	.006	3	.025	1	.004	1	-5.963e-6	15	NC	5	NC	2
430			min	-.007	2	-.017	3	-.001	3	-1.819e-4	1	842.745	1	4876.024	1
431		7	max	.006	3	.031	1	.002	1	-9.531e-6	15	NC	5	NC	2
432			min	-.007	2	-.021	3	-.002	3	-2.881e-4	1	759.795	1	6430.49	1
433		8	max	.006	3	.035	1	0	10	-1.31e-5	15	NC	5	NC	1
434			min	-.007	2	-.023	3	-.002	3	-3.942e-4	1	709.659	1	NC	1
435		9	max	.006	3	.037	1	0	10	-1.667e-5	15	NC	5	NC	1
436			min	-.007	2	-.025	3	-.004	1	-5.003e-4	1	682.359	1	NC	1
437		10	max	.006	3	.038	1	0	15	-2.024e-5	15	NC	5	NC	1
438			min	-.008	2	-.025	3	-.008	1	-6.064e-4	1	673.298	1	NC	1
439		11	max	.006	3	.037	1	0	15	-2.38e-5	15	NC	5	NC	2
440			min	-.008	2	-.024	3	-.011	1	-7.125e-4	1	681.092	1	6899.551	1
441		12	max	.006	3	.035	1	0	15	-2.737e-5	15	NC	5	NC	2
442			min	-.008	2	-.022	3	-.014	1	-8.187e-4	1	706.992	1	4630.71	1
443		13	max	.006	3	.03	1	0	15	-3.094e-5	15	NC	5	NC	2
444			min	-.008	2	-.019	3	-.017	1	-9.248e-4	1	755.413	1	3656.723	1
445		14	max	.006	3	.024	1	0	15	-3.451e-5	15	NC	5	NC	3
446			min	-.008	2	-.015	3	-.018	1	-1.031e-3	1	836.019	1	3209.573	1
447		15	max	.006	3	.016	1	0	15	-3.808e-5	15	NC	5	NC	3
448			min	-.008	2	-.01	3	-.019	1	-1.137e-3	1	969.524	1	3083.376	1
449		16	max	.006	3	.007	1	0	15	-4.068e-5	15	NC	5	NC	3
450			min	-.008	2	-.004	3	-.017	1	-1.215e-3	1	1205.32	1	3269.305	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.006	3	.002	3	0	15	-1.425e-5	12	NC	4	NC	2
452			min	-.008	2	-.005	1	-.014	1	-6.304e-4	1	1691.799	1	4002.856	1
453		18	max	.006	3	.01	3	0	15	3.876e-3	3	NC	4	NC	2
454			min	-.008	2	-.018	1	-.009	1	-1.145e-2	1	3260.917	1	6667.974	1
455		19	max	.006	3	.018	3	0	3	7.844e-3	3	NC	1	NC	1
456			min	-.008	2	-.033	1	-.002	1	-2.248e-2	1	NC	1	NC	1
457	M13	1	max	.008	1	.023	3	.006	3	3.971e-3	3	NC	1	NC	1
458			min	0	3	-.03	1	-.007	2	-5.296e-3	1	NC	1	NC	1
459		2	max	.007	1	.21	3	.056	1	4.809e-3	3	NC	5	NC	3
460			min	0	3	-.249	1	.001	10	-6.448e-3	1	934.129	1	3361.316	1
461		3	max	.007	1	.363	3	.141	1	5.647e-3	3	NC	5	NC	3
462			min	0	3	-.427	1	.005	15	-7.601e-3	1	513.956	1	1395.468	1
463		4	max	.007	1	.458	3	.214	1	6.485e-3	3	NC	5	NC	3
464			min	0	3	-.539	1	.007	15	-8.754e-3	1	400.981	1	931.764	1
465		5	max	.007	1	.485	3	.25	1	7.323e-3	3	NC	15	NC	3
466			min	0	3	-.572	1	.009	15	-9.907e-3	1	376.938	1	800.368	1
467		6	max	.007	1	.445	3	.238	1	8.161e-3	3	NC	5	NC	3
468			min	-.001	3	-.526	1	.008	15	-1.106e-2	1	411.476	1	837.987	1
469		7	max	.007	1	.351	3	.182	1	9.e-3	3	NC	5	NC	3
470			min	-.001	3	-.418	1	.006	15	-1.221e-2	1	525.565	1	1087.867	1
471		8	max	.007	1	.23	3	.098	1	9.838e-3	3	NC	5	NC	3
472			min	-.001	3	-.279	1	0	10	-1.336e-2	1	821.924	1	1970.489	1
473		9	max	.006	1	.12	3	.017	3	1.068e-2	3	NC	5	NC	2
474			min	-.001	3	-.15	1	-.009	10	-1.452e-2	1	1700.72	1	9559.796	1
475		10	max	.006	1	.069	3	.018	3	1.151e-2	3	NC	4	NC	1
476			min	-.001	3	-.092	1	-.024	2	-1.567e-2	1	3310.208	1	NC	1
477		11	max	.006	1	.12	3	.022	1	1.068e-2	3	NC	5	NC	2
478			min	-.001	3	-.15	1	-.009	10	-1.452e-2	1	1700.721	1	7535.666	1
479		12	max	.006	1	.23	3	.108	1	9.838e-3	3	NC	5	NC	3
480			min	-.001	3	-.278	1	0	10	-1.336e-2	1	821.925	1	1806.67	1
481		13	max	.006	1	.351	3	.194	1	9.001e-3	3	NC	5	NC	5
482			min	-.001	3	-.418	1	.007	15	-1.221e-2	1	525.565	1	1026.337	1
483		14	max	.006	1	.445	3	.25	1	8.163e-3	3	NC	5	NC	5
484			min	-.001	3	-.526	1	.009	15	-1.106e-2	1	411.476	1	799.96	1
485		15	max	.006	1	.485	3	.26	1	7.325e-3	3	NC	15	NC	5
486			min	-.001	3	-.571	1	.009	15	-9.906e-3	1	376.938	1	768.258	1
487		16	max	.006	1	.458	3	.223	1	6.487e-3	3	NC	5	NC	3
488			min	-.001	3	-.539	1	.008	15	-8.753e-3	1	400.981	1	895.916	1
489		17	max	.006	1	.363	3	.147	1	5.649e-3	3	NC	5	NC	3
490			min	-.001	3	-.427	1	.005	15	-7.6e-3	1	513.956	1	1338.956	1
491		18	max	.005	1	.21	3	.059	1	4.811e-3	3	NC	5	NC	3
492			min	-.001	3	-.249	1	.001	10	-6.447e-3	1	934.13	1	3195.059	1
493		19	max	.005	1	.023	3	.006	3	3.974e-3	3	NC	1	NC	1
494			min	-.001	3	-.03	1	-.007	2	-5.294e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.018	3	.006	3	5.519e-3	1	NC	1	NC	1
496			min	0	3	-.033	1	-.008	2	-3.016e-3	3	NC	1	NC	1
497		2	max	.002	1	.104	3	.06	1	6.759e-3	1	NC	5	NC	3
498			min	0	3	-.279	1	.001	10	-3.624e-3	3	829.283	1	3124.035	1
499		3	max	.002	1	.174	3	.149	1	7.999e-3	1	NC	5	NC	3
500			min	0	3	-.48	1	.005	15	-4.231e-3	3	456.295	1	1324.115	1
501		4	max	.002	1	.219	3	.223	1	9.239e-3	1	NC	5	NC	3
502			min	0	3	-.606	1	.008	15	-4.839e-3	3	356.031	1	891.535	1
503		5	max	.003	1	.233	3	.26	1	1.048e-2	1	NC	15	NC	5
504			min	0	3	-.642	1	.009	15	-5.447e-3	3	334.742	1	768.152	1
505		6	max	.003	1	.216	3	.248	1	1.172e-2	1	NC	5	NC	5
506			min	0	3	-.591	1	.009	15	-6.054e-3	3	365.525	1	803.708	1
507		7	max	.003	1	.176	3	.191	1	1.296e-2	1	NC	5	NC	5

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508			min	0	3	-.469	1	.007	15	-6.662e-3	3	467.146	1	1037.861	1
509		8	max	.003	1	.123	3	.105	1	1.42e-2	1	NC	5	NC	3
510			min	0	3	-.312	1	0	10	-7.269e-3	3	731.534	1	1850.362	1
511		9	max	.003	1	.074	3	.021	3	1.544e-2	1	NC	5	NC	2
512			min	0	3	-.167	1	-.009	10	-7.877e-3	3	1519.434	1	8207.608	1
513		10	max	.003	1	.051	3	.018	3	1.668e-2	1	NC	4	NC	1
514			min	0	3	-.101	1	-.025	2	-8.485e-3	3	2977.857	1	NC	1
515		11	max	.003	1	.074	3	.018	3	1.544e-2	1	NC	5	NC	2
516			min	0	3	-.167	1	-.009	10	-7.877e-3	3	1519.434	1	8874.168	1
517		12	max	.003	1	.123	3	.101	1	1.42e-2	1	NC	5	NC	3
518			min	0	3	-.312	1	0	10	-7.269e-3	3	731.535	1	1911.177	1
519		13	max	.003	1	.176	3	.186	1	1.296e-2	1	NC	5	NC	3
520			min	0	3	-.469	1	.007	15	-6.661e-3	3	467.146	1	1063.953	1
521		14	max	.004	1	.216	3	.243	1	1.172e-2	1	NC	5	NC	3
522			min	0	3	-.591	1	.008	15	-6.053e-3	3	365.525	1	822.172	1
523		15	max	.004	1	.233	3	.254	1	1.048e-2	1	NC	15	NC	3
524			min	0	3	-.642	1	.009	15	-5.445e-3	3	334.742	1	786.044	1
525		16	max	.004	1	.219	3	.218	1	9.241e-3	1	NC	5	NC	3
526			min	0	3	-.606	1	.007	15	-4.837e-3	3	356.031	1	914.517	1
527		17	max	.004	1	.174	3	.144	1	8.001e-3	1	NC	5	NC	3
528			min	0	3	-.48	1	.005	15	-4.229e-3	3	456.296	1	1365.903	1
529		18	max	.004	1	.104	3	.057	1	6.762e-3	1	NC	5	NC	3
530			min	0	3	-.279	1	.001	10	-3.621e-3	3	829.283	1	3265.619	1
531		19	max	.004	1	.018	3	.006	3	5.522e-3	1	NC	1	NC	1
532			min	0	3	-.033	1	-.008	2	-3.013e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	1	0	1	3.166e-4	3	NC	1	NC	1
534			min	0	1	0	1	0	1	-1.151e-4	2	NC	1	NC	1
535		2	max	0	1	-.007	15	.001	1	8.209e-4	3	NC	15	NC	1
536			min	0	10	-.03	4	0	3	-8.074e-4	1	3626.105	4	NC	1
537		3	max	0	1	-.014	15	.004	1	1.325e-3	3	7849.751	15	NC	1
538			min	0	10	-.058	4	-.003	3	-1.54e-3	1	1845.201	4	NC	1
539		4	max	0	1	-.02	15	.008	1	1.829e-3	3	5385.388	15	NC	3
540			min	0	10	-.085	4	-.006	3	-2.273e-3	1	1265.916	4	8606.436	1
541		5	max	0	1	-.026	15	.014	1	2.334e-3	3	4202.273	15	NC	4
542			min	0	10	-.109	4	-.01	3	-3.006e-3	1	987.807	4	5638.362	1
543		6	max	0	1	-.03	15	.019	1	2.838e-3	3	3536.657	15	NC	4
544			min	0	10	-.129	4	-.015	3	-3.739e-3	1	831.344	4	4100.254	1
545		7	max	0	1	-.034	15	.025	1	3.342e-3	3	3136.376	15	NC	4
546			min	0	10	-.146	4	-.019	3	-4.472e-3	1	737.252	4	3202.311	1
547		8	max	0	1	-.037	15	.031	1	3.847e-3	3	2896.148	15	NC	4
548			min	0	10	-.158	4	-.024	3	-5.205e-3	1	680.783	4	2638.468	1
549		9	max	0	1	-.039	15	.037	1	4.351e-3	3	2766.843	15	NC	4
550			min	0	10	-.166	4	-.028	3	-5.938e-3	1	650.388	4	2269.751	1
551		10	max	0	1	-.039	15	.041	1	4.855e-3	3	2725.937	15	NC	4
552			min	0	10	-.168	4	-.032	3	-6.671e-3	1	640.772	4	2026.458	1
553		11	max	0	1	-.039	15	.044	1	5.36e-3	3	2766.843	15	NC	5
554			min	0	10	-.166	4	-.034	3	-7.404e-3	1	650.388	4	1872.02	1
555		12	max	0	1	-.037	15	.045	1	5.864e-3	3	2896.148	15	NC	5
556			min	0	10	-.159	4	-.035	3	-8.137e-3	1	680.783	4	1788.275	1
557		13	max	0	1	-.034	15	.045	1	6.368e-3	3	3136.376	15	NC	5
558			min	0	10	-.147	4	-.034	3	-8.87e-3	1	737.252	4	1769.884	1
559		14	max	0	1	-.03	15	.041	1	6.873e-3	3	3536.657	15	NC	5
560			min	0	10	-.13	4	-.032	3	-9.603e-3	1	831.344	4	1824.469	1
561		15	max	0	1	-.026	15	.035	1	7.377e-3	3	4202.273	15	NC	4
562			min	0	10	-.11	4	-.027	3	-1.034e-2	1	987.807	4	1980.296	1
563		16	max	0	1	-.02	15	.026	1	7.881e-3	3	5385.388	15	NC	4
564			min	0	10	-.086	4	-.02	3	-1.107e-2	1	1265.916	4	2314.226	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	1	-.014	15	.014	1	8.385e-3	3	7849.751	15	NC	4
566		min	0	10	-.059	4	-.009	3	-1.18e-2	1	1845.201	4	3067.491	1
567	18	max	0	1	-.007	15	.004	3	8.89e-3	3	NC	15	NC	4
568		min	0	10	-.031	4	-.008	2	-1.253e-2	1	3626.105	4	5460.522	1
569	19	max	.001	1	.004	3	.021	3	9.394e-3	3	NC	1	NC	1
570		min	0	10	-.005	1	-.026	2	-1.327e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.007	3	3.164e-3	3	NC	1	NC	1
572		min	-.001	1	-.002	1	-.008	2	-4.023e-3	1	NC	1	NC	1
573	2	max	0	10	-.007	15	.006	1	3.023e-3	3	NC	15	NC	2
574		min	-.001	1	-.03	4	0	10	-3.828e-3	1	3626.105	4	8653.602	1
575	3	max	0	10	-.014	15	.016	1	2.882e-3	3	7849.751	15	NC	3
576		min	0	1	-.059	4	-.004	3	-3.632e-3	1	1845.201	4	4893.606	1
577	4	max	0	10	-.02	15	.024	1	2.741e-3	3	5385.388	15	NC	4
578		min	0	1	-.085	4	-.008	3	-3.436e-3	1	1265.916	4	3719.704	1
579	5	max	0	10	-.026	15	.029	1	2.6e-3	3	4202.273	15	NC	4
580		min	0	1	-.109	4	-.01	3	-3.241e-3	1	987.807	4	3210.274	1
581	6	max	0	10	-.03	15	.031	1	2.46e-3	3	3536.657	15	NC	4
582		min	0	1	-.13	4	-.012	3	-3.045e-3	1	831.344	4	2986.837	1
583	7	max	0	10	-.034	15	.032	1	2.319e-3	3	3136.376	15	NC	4
584		min	0	1	-.146	4	-.012	3	-2.849e-3	1	737.252	4	2930.694	1
585	8	max	0	10	-.037	15	.032	1	2.178e-3	3	2896.148	15	NC	4
586		min	0	1	-.158	4	-.012	3	-2.654e-3	1	680.783	4	3001.099	1
587	9	max	0	10	-.039	15	.03	1	2.037e-3	3	2766.843	15	NC	4
588		min	0	1	-.165	4	-.012	3	-2.458e-3	1	650.388	4	3192.256	1
589	10	max	0	10	-.039	15	.027	1	1.896e-3	3	2725.937	15	NC	4
590		min	0	1	-.168	4	-.011	3	-2.262e-3	1	640.772	4	3523.276	1
591	11	max	0	10	-.039	15	.024	1	1.756e-3	3	2766.843	15	NC	4
592		min	0	1	-.165	4	-.009	3	-2.067e-3	1	650.388	4	4042.246	1
593	12	max	0	10	-.037	15	.02	1	1.615e-3	3	2896.148	15	NC	3
594		min	0	1	-.158	4	-.008	3	-1.871e-3	1	680.783	4	4844.73	1
595	13	max	0	10	-.034	15	.015	1	1.474e-3	3	3136.376	15	NC	3
596		min	0	1	-.146	4	-.006	3	-1.675e-3	1	737.252	4	6121.272	1
597	14	max	0	10	-.03	15	.011	1	1.333e-3	3	3536.657	15	NC	2
598		min	0	1	-.129	4	-.004	3	-1.48e-3	1	831.344	4	8283.493	1
599	15	max	0	10	-.026	15	.007	1	1.192e-3	3	4202.273	15	NC	1
600		min	0	1	-.109	4	-.002	3	-1.284e-3	1	987.807	4	NC	1
601	16	max	0	10	-.02	15	.004	1	1.052e-3	3	5385.388	15	NC	1
602		min	0	1	-.085	4	-.001	3	-1.088e-3	1	1265.916	4	NC	1
603	17	max	0	10	-.014	15	.001	1	9.107e-4	3	7849.751	15	NC	1
604		min	0	1	-.058	4	0	3	-8.927e-4	1	1845.201	4	NC	1
605	18	max	0	10	-.007	15	0	4	7.699e-4	3	NC	15	NC	1
606		min	0	1	-.03	4	0	2	-7.262e-4	2	3626.105	4	NC	1
607	19	max	0	1	0	1	0	1	6.291e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.634e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.