

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	25° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 25°  
Maximum Height Above Grade = 3 ft

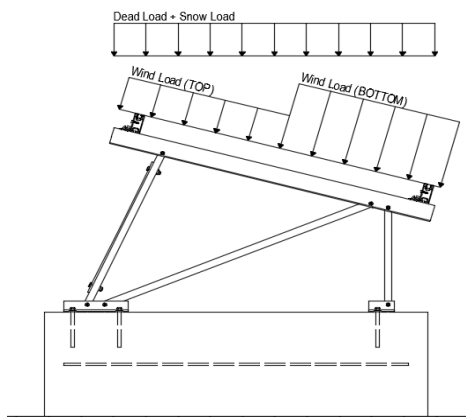
### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	18.56 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.82	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	100 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II
Peak Velocity Pressure, $q_z$ =	15.70 psf	Including the gust factor, $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25	ASCE 7, Section 12.8.1.3: A maximum $S_S$ of 1.5 may be used to calculate the base shear, $C_s$ , of structures under five stories and with a period, $T$ , of 0.5 or less. Therefore, a $S_{ds}$ of 1.0 was used to calculate $C_s$ .
$S_{DS}$ =	1.67	$C_s$ = 0.8	
$S_1$ =	1.00	$\rho$ = 1.3	
$S_{D1}$ =	1.00	$\Omega$ = 1.25	
$T_a$ =	0.04	$C_d$ = 1.25	

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

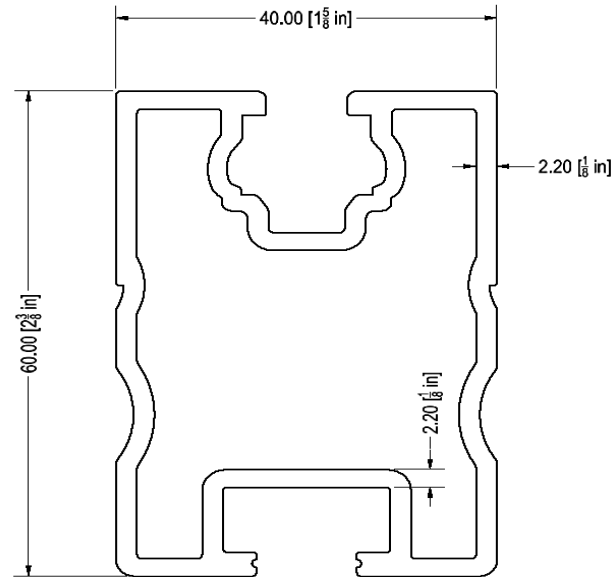
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

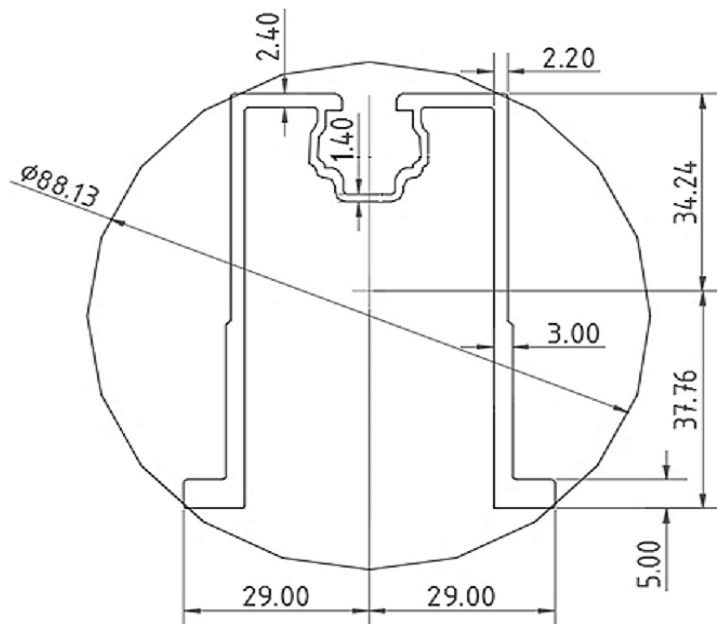
Purlin Type =	<b>ProfiPlus</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	72 in
$\Phi F_{ty}$ STRONG-AXIS =	28.91 ksi
$\Phi F_{ty}$ WEAK-AXIS =	28.47 ksi
$S_y$ =	0.51 in <sup>3</sup>
$S_x$ =	0.37 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.60 in <sup>4</sup>
$I_x$ =	0.29 in <sup>4</sup>
$A$ =	0.90 in <sup>2</sup>
$g$ =	1.08 lbs/ft
$M_y$ =	0.634 k-ft
$M_z$ =	0.135 k-ft
$M_{y \text{ allowable}}$ =	1.230 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	<b>67%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.36 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.555 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.293 k
$M_{y \text{ allowable}}$ =	1.441 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>41%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.041 k-ft
$P_n$ =	0.217 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>11%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.474 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>12%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.909 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	<b>16%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.005 k-ft
$P_n$ =	0.211 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>13%</b>



A cross brace kit is required every 15 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

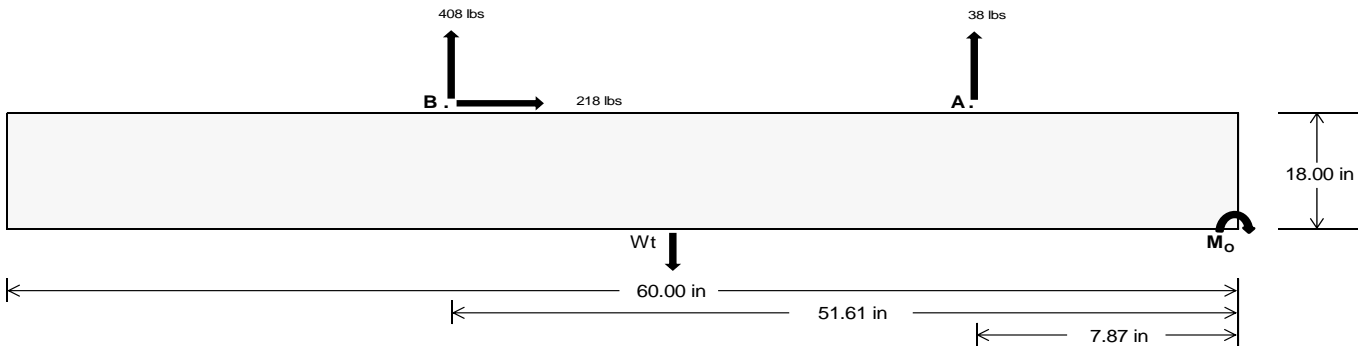
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>164.14</b>	<b>1699.85</b>	k
Compressive Load =	<b>1442.58</b>	<b>1232.54</b>	k
Lateral Load =	<b>33.41</b>	<b>906.00</b>	k
Moment (Weak Axis) =	<b>0.05</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 25277.4$  in-lbs  
Resisting Force Required = 842.58 lbs  
S.F. = 1.67  
Weight Required = 1404.30 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 217.72 lbs  
Friction = 0.4  
Weight Required = 544.29 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 217.72 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	524 lbs	524 lbs	524 lbs	524 lbs	469 lbs	469 lbs	469 lbs	469 lbs	701 lbs	701 lbs	701 lbs	701 lbs	-77 lbs	-77 lbs	-77 lbs	-77 lbs
$F_B$	376 lbs	376 lbs	376 lbs	376 lbs	511 lbs	511 lbs	511 lbs	511 lbs	633 lbs	633 lbs	633 lbs	633 lbs	-816 lbs	-816 lbs	-816 lbs	-816 lbs
$F_V$	53 lbs	53 lbs	53 lbs	53 lbs	392 lbs	392 lbs	392 lbs	392 lbs	330 lbs	330 lbs	330 lbs	330 lbs	-435 lbs	-435 lbs	-435 lbs	-435 lbs
$P_{total}$	2893 lbs	2984 lbs	3075 lbs	3165 lbs	2973 lbs	3064 lbs	3154 lbs	3245 lbs	3328 lbs	3419 lbs	3510 lbs	3600 lbs	304 lbs	358 lbs	412 lbs	467 lbs
$M$	369 lbs-ft	369 lbs-ft	369 lbs-ft	369 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	648 lbs-ft	648 lbs-ft	648 lbs-ft	648 lbs-ft	675 lbs-ft	675 lbs-ft	675 lbs-ft	675 lbs-ft
$e$	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.19 ft	0.19 ft	0.19 ft	0.18 ft	2.22 ft	1.89 ft	1.64 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	267.4 psf	265.2 psf	263.2 psf	261.4 psf	254.6 psf	253.0 psf	251.5 psf	250.2 psf	278.2 psf	275.6 psf	273.2 psf	270.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	363.9 psf	357.6 psf	351.7 psf	346.4 psf	394.0 psf	386.4 psf	379.3 psf	372.8 psf	447.9 psf	437.9 psf	428.7 psf	420.3 psf	398.8 psf	202.6 psf	159.2 psf	141.7 psf

Maximum Bearing Pressure = 448 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

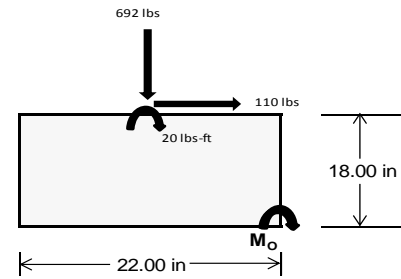
### Overturning Check

$M_o = 449.5 \text{ ft-lbs}$   
 Resisting Force Required = 490.38 lbs  
 S.F. = 1.67  
 Weight Required = 817.31 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	128 lbs	123 lbs	71 lbs	295 lbs	692 lbs	251 lbs	78 lbs	-7 lbs	24 lbs
$F_v$	18 lbs	146 lbs	18 lbs	12 lbs	110 lbs	14 lbs	18 lbs	146 lbs	18 lbs
$P_{total}$	2596 lbs	2592 lbs	2540 lbs	2645 lbs	3042 lbs	2601 lbs	799 lbs	714 lbs	746 lbs
$M$	52 lbs-ft	246 lbs-ft	55 lbs-ft	34 lbs-ft	185 lbs-ft	42 lbs-ft	53 lbs-ft	246 lbs-ft	55 lbs-ft
$e$	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.02 ft	0.07 ft	0.34 ft	0.07 ft
$L/6$	0.31 ft	1.64 ft	1.79 ft	1.81 ft	1.71 ft	1.80 ft	1.70 ft	1.15 ft	1.69 ft
$f_{min}$	264.6 sqft	195.0 sqft	257.5 sqft	276.2 sqft	265.7 sqft	268.6 sqft	68.5 sqft	-9.7 sqft	61.8 sqft
$f_{max}$	301.8 psf	370.5 psf	296.6 psf	300.8 psf	398.0 psf	298.9 psf	106.0 psf	165.6 psf	100.8 psf



Maximum Bearing Pressure = 398 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



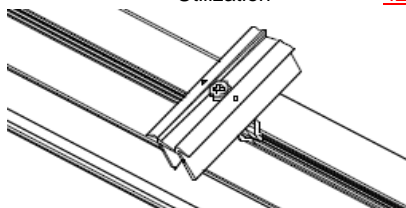
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

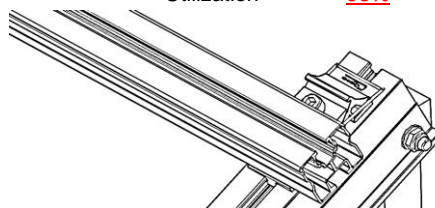
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.516 k
Allowable Uplift =	1.214 k
Utilization =	<u>42%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

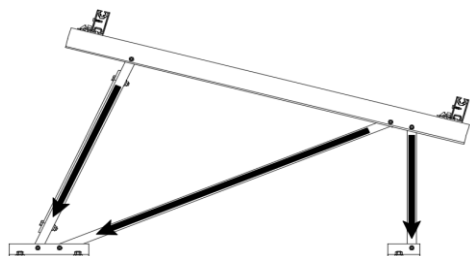
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.110 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Diagonal Strut

Maximum Axial Load =	0.474 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



#### Rear Strut

Maximum Axial Load =	1.166 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.211 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	30.83 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.617 in
Max Drift, $\Delta_{MAX}$ =	0.086 in
	<u>0.086 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 72.00 \text{ in}$$

$$J = 0.255$$

$$187.484$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 72.00 \text{ in}$$

$$J = 0.255$$

$$194.691$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.8$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.230 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.08 \\
 &23.7085 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.08 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.441 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.5514 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.7972 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 11.5927 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

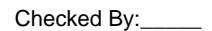
$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 11.59 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.82 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





## RISA-3D Version 13.0.0 \\\...\\PVMini 60 Cell 1V 25° 100mph 30psf 6ft 7-05.r3dPage 21



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	283.093	1	.03	2	.428	1	0	10	0	4	0	15
30			min	-359.122	3	-.029	3	-.407	5	0	1	0	3	0	6
31		16	max	283.21	1	-.006	2	.428	1	0	10	0	1	0	15
32			min	-359.035	3	-.056	3	-.513	5	0	1	0	3	0	6
33		17	max	283.326	1	-.022	15	.428	1	0	10	.001	1	0	15
34			min	-358.947	3	-.091	4	-.618	5	0	1	0	3	0	6
35		18	max	283.442	1	-.033	15	.428	1	0	10	.001	1	0	15
36			min	-358.86	3	-.137	4	-.724	5	0	1	0	3	0	6
37		19	max	283.559	1	-.044	15	.428	1	0	10	.001	1	0	15
38			min	-358.773	3	-.182	4	-.829	5	0	1	0	3	0	6
39	M3	1	max	118.108	2	1.775	6	-.029	12	0	5	.001	1	0	6
40			min	-127.599	3	.417	15	-1.392	4	0	1	0	12	0	15
41		2	max	118.039	2	1.598	6	-.029	12	0	5	.001	1	0	2
42			min	-127.65	3	.375	15	-1.259	4	0	1	0	12	0	15
43		3	max	117.971	2	1.421	6	-.029	12	0	5	.001	1	0	2
44			min	-127.702	3	.333	15	-1.125	4	0	1	0	15	0	3
45		4	max	117.902	2	1.244	6	-.029	12	0	5	.001	1	0	15
46			min	-127.753	3	.292	15	-.991	4	0	1	0	5	0	4
47		5	max	117.834	2	1.067	6	-.029	12	0	5	0	1	0	15
48			min	-127.805	3	.25	15	-.858	4	0	1	0	5	0	4
49		6	max	117.765	2	.889	6	-.029	12	0	5	0	1	0	15
50			min	-127.856	3	.208	15	-.724	4	0	1	0	5	0	4
51		7	max	117.696	2	.712	6	-.029	12	0	5	0	1	0	15
52			min	-127.907	3	.167	15	-.59	4	0	1	0	5	0	4
53		8	max	117.628	2	.535	6	-.029	12	0	5	0	1	0	15
54			min	-127.959	3	.125	15	-.457	4	0	1	0	5	-.001	4
55		9	max	117.559	2	.358	6	-.029	12	0	5	0	1	0	15
56			min	-128.01	3	.083	15	-.401	1	0	1	0	5	-.001	4
57		10	max	117.491	2	.181	6	-.029	12	0	5	0	1	0	15
58			min	-128.062	3	.042	15	-.401	1	0	1	0	5	-.001	4
59		11	max	117.422	2	.026	2	.029	5	0	5	0	1	0	15
60			min	-128.113	3	-.021	3	-.401	1	0	1	0	5	-.001	4
61		12	max	117.353	2	-.042	15	.163	5	0	5	0	1	0	15
62			min	-128.165	3	-.174	4	-.401	1	0	1	0	5	-.001	4
63		13	max	117.285	2	-.083	15	.296	5	0	5	0	1	0	15
64			min	-128.216	3	-.351	4	-.401	1	0	1	0	5	-.001	4
65		14	max	117.216	2	-.125	15	.43	5	0	5	0	1	0	15
66			min	-128.268	3	-.528	4	-.401	1	0	1	0	5	-.001	4
67		15	max	117.148	2	-.166	15	.564	5	0	5	0	1	0	15
68			min	-128.319	3	-.706	4	-.401	1	0	1	0	5	0	4
69		16	max	117.079	2	-.208	15	.697	5	0	5	0	12	0	15
70			min	-128.371	3	-.883	4	-.401	1	0	1	0	4	0	4
71		17	max	117.01	2	-.25	15	.831	5	0	5	0	12	0	15
72			min	-128.422	3	-1.06	4	-.401	1	0	1	0	4	0	4
73		18	max	116.942	2	-.291	15	.964	5	0	5	0	12	0	15
74			min	-128.473	3	-1.237	4	-.401	1	0	1	0	1	0	4
75		19	max	116.873	2	-.333	15	1.098	5	0	5	0	5	0	1
76			min	-128.525	3	-1.414	4	-.401	1	0	1	0	1	0	1
77	M4	1	max	399.528	1	0	1	-.12	10	0	1	0	5	0	1
78			min	-30.383	3	0	1	-24.74	4	0	1	0	2	0	1
79		2	max	399.593	1	0	1	-.12	10	0	1	0	12	0	1
80			min	-30.335	3	0	1	-24.796	4	0	1	-.002	4	0	1
81		3	max	399.658	1	0	1	-.12	10	0	1	0	12	0	1
82			min	-30.286	3	0	1	-24.852	4	0	1	-.004	4	0	1
83		4	max	399.722	1	0	1	-.12	10	0	1	0	12	0	1
84			min	-30.238	3	0	1	-24.908	4	0	1	-.007	4	0	1
85		5	max	399.787	1	0	1	-.12	10	0	1	0	12	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-30.189	3	0	1	-24.964	4	0	1	-.009	4	0	1
87		6	max	399.852	1	0	1	-.12	10	0	1	0	12	0	1
88			min	-30.141	3	0	1	-25.02	4	0	1	-.011	4	0	1
89		7	max	399.917	1	0	1	-.12	10	0	1	0	12	0	1
90			min	-30.092	3	0	1	-25.076	4	0	1	-.013	4	0	1
91		8	max	399.981	1	0	1	-.12	10	0	1	0	10	0	1
92			min	-30.044	3	0	1	-25.132	4	0	1	-.016	4	0	1
93		9	max	400.046	1	0	1	-.12	10	0	1	0	10	0	1
94			min	-29.995	3	0	1	-25.188	4	0	1	-.018	4	0	1
95		10	max	400.111	1	0	1	-.12	10	0	1	0	10	0	1
96			min	-29.947	3	0	1	-25.244	4	0	1	-.02	4	0	1
97		11	max	400.175	1	0	1	-.12	10	0	1	0	10	0	1
98			min	-29.898	3	0	1	-25.3	4	0	1	-.022	4	0	1
99		12	max	400.24	1	0	1	-.12	10	0	1	0	10	0	1
100			min	-29.85	3	0	1	-25.356	4	0	1	-.025	4	0	1
101		13	max	400.305	1	0	1	-.12	10	0	1	0	10	0	1
102			min	-29.801	3	0	1	-25.412	4	0	1	-.027	4	0	1
103		14	max	400.37	1	0	1	-.12	10	0	1	0	10	0	1
104			min	-29.752	3	0	1	-25.469	4	0	1	-.029	4	0	1
105		15	max	400.434	1	0	1	-.12	10	0	1	0	10	0	1
106			min	-29.704	3	0	1	-25.525	4	0	1	-.031	4	0	1
107		16	max	400.499	1	0	1	-.12	10	0	1	0	10	0	1
108			min	-29.655	3	0	1	-25.581	4	0	1	-.034	4	0	1
109		17	max	400.564	1	0	1	-.12	10	0	1	0	10	0	1
110			min	-29.607	3	0	1	-25.637	4	0	1	-.036	4	0	1
111		18	max	400.628	1	0	1	-.12	10	0	1	0	10	0	1
112			min	-29.558	3	0	1	-25.693	4	0	1	-.038	4	0	1
113		19	max	400.693	1	0	1	-.12	10	0	1	0	10	0	1
114			min	-29.51	3	0	1	-25.749	4	0	1	-.041	4	0	1
115	M6	1	max	907.19	1	.629	6	1.096	4	0	3	0	3	0	1
116			min	-1165.83	3	.142	15	-.177	3	0	5	0	1	0	1
117		2	max	907.307	1	.584	6	.99	4	0	3	0	4	0	15
118			min	-1165.743	3	.132	15	-.177	3	0	5	0	1	0	6
119		3	max	907.423	1	.538	6	.885	4	0	3	0	4	0	15
120			min	-1165.656	3	.121	15	-.177	3	0	5	0	1	0	6
121		4	max	907.54	1	.492	6	.779	4	0	3	0	4	0	15
122			min	-1165.569	3	.11	15	-.177	3	0	5	0	10	0	6
123		5	max	907.656	1	.45	2	.674	4	0	3	0	4	0	15
124			min	-1165.481	3	.099	15	-.177	3	0	5	0	10	0	6
125		6	max	907.772	1	.414	2	.568	4	0	3	0	4	0	15
126			min	-1165.394	3	.089	15	-.177	3	0	5	0	3	0	6
127		7	max	907.889	1	.379	2	.463	4	0	3	0	4	0	15
128			min	-1165.307	3	.078	15	-.177	3	0	5	0	3	0	6
129		8	max	908.005	1	.343	2	.357	4	0	3	0	4	0	15
130			min	-1165.219	3	.065	12	-.177	3	0	5	0	3	0	6
131		9	max	908.122	1	.308	2	.252	4	0	3	0	4	0	15
132			min	-1165.132	3	.047	12	-.177	3	0	5	0	3	0	2
133		10	max	908.238	1	.272	2	.197	1	0	3	0	4	0	15
134			min	-1165.045	3	.03	12	-.177	3	0	5	0	3	0	2
135		11	max	908.355	1	.236	2	.197	1	0	3	.001	4	0	15
136			min	-1164.957	3	.012	3	-.177	3	0	5	0	3	0	2
137		12	max	908.471	1	.201	2	.197	1	0	3	.001	4	0	15
138			min	-1164.87	3	-.015	3	-.177	3	0	5	0	3	0	2
139		13	max	908.587	1	.165	2	.197	1	0	3	0	4	0	15
140			min	-1164.783	3	-.042	3	-.211	5	0	5	0	3	0	2
141		14	max	908.704	1	.13	2	.197	1	0	3	0	4	0	15
142			min	-1164.696	3	-.068	3	-.317	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	908.82	1	.094	2	.197	1	0	3	0	4	0	12
144		min	-1164.608	3	-.095	3	-.422	5	0	5	0	3	0	2
145	16	max	908.937	1	.058	2	.197	1	0	3	0	4	0	12
146		min	-1164.521	3	-.122	3	-.528	5	0	5	0	3	0	2
147	17	max	909.053	1	.023	2	.197	1	0	3	0	4	0	12
148		min	-1164.434	3	-.148	3	-.633	5	0	5	0	3	0	2
149	18	max	909.169	1	-.013	2	.197	1	0	3	0	4	0	12
150		min	-1164.346	3	-.175	3	-.739	5	0	5	0	3	0	2
151	19	max	909.286	1	-.048	2	.197	1	0	3	0	1	0	12
152		min	-1164.259	3	-.202	3	-.844	5	0	5	0	3	0	2
153	M7	1	max	473.885	2	1.789	.01	3	0	1	0	4	0	2
154		min	-388.375	3	.425	15	-1.364	5	0	3	0	3	0	12
155	2	max	473.816	2	1.612	4	.01	3	0	1	0	4	0	2
156		min	-388.426	3	.383	15	-1.231	5	0	3	0	3	0	3
157	3	max	473.747	2	1.435	4	.01	3	0	1	0	1	0	2
158		min	-388.478	3	.342	15	-1.097	5	0	3	0	3	0	3
159	4	max	473.679	2	1.257	4	.01	3	0	1	0	1	0	2
160		min	-388.529	3	.3	15	-.963	5	0	3	0	3	0	3
161	5	max	473.61	2	1.08	4	.01	3	0	1	0	1	0	15
162		min	-388.58	3	.258	15	-.83	5	0	3	0	5	0	3
163	6	max	473.542	2	.903	4	.01	3	0	1	0	1	0	15
164		min	-388.632	3	.217	15	-.696	5	0	3	0	5	0	6
165	7	max	473.473	2	.726	4	.01	3	0	1	0	1	0	15
166		min	-388.683	3	.175	15	-.562	5	0	3	0	5	0	6
167	8	max	473.404	2	.549	4	.01	3	0	1	0	1	0	15
168		min	-388.735	3	.133	15	-.429	5	0	3	0	5	-.001	6
169	9	max	473.336	2	.371	4	.01	3	0	1	0	1	0	15
170		min	-388.786	3	.088	12	-.295	5	0	3	0	5	-.001	6
171	10	max	473.267	2	.222	2	.01	3	0	1	0	1	0	15
172		min	-388.838	3	.019	12	-.162	5	0	3	0	5	-.001	6
173	11	max	473.199	2	.084	2	.01	3	0	1	0	1	0	15
174		min	-388.889	3	-.08	3	-.028	5	0	3	0	5	-.001	6
175	12	max	473.13	2	-.033	15	.107	4	0	1	0	1	0	15
176		min	-388.941	3	-.183	3	-.01	2	0	3	0	5	-.001	6
177	13	max	473.061	2	-.075	15	.24	4	0	1	0	1	0	15
178		min	-388.992	3	-.338	6	-.01	2	0	3	0	5	-.001	6
179	14	max	472.993	2	-.117	15	.374	4	0	1	0	1	0	15
180		min	-389.044	3	-.515	6	-.01	2	0	3	0	5	-.001	6
181	15	max	472.924	2	-.158	15	.507	4	0	1	0	1	0	15
182		min	-389.095	3	-.692	6	-.01	2	0	3	0	5	0	6
183	16	max	472.856	2	-.2	15	.641	4	0	1	0	1	0	15
184		min	-389.146	3	-.869	6	-.01	2	0	3	0	5	0	6
185	17	max	472.787	2	-.241	15	.775	4	0	1	0	1	0	15
186		min	-389.198	3	-1.047	6	-.01	2	0	3	0	5	0	6
187	18	max	472.718	2	-.283	15	.908	4	0	1	0	1	0	15
188		min	-389.249	3	-1.224	6	-.01	2	0	3	0	5	0	6
189	19	max	472.65	2	-.325	15	1.042	4	0	1	0	1	0	1
190		min	-389.301	3	-1.401	6	-.01	2	0	3	0	3	0	1
191	M8	1	max	1108.511	1	0	.656	1	0	1	0	4	0	1
192		min	-127.132	3	0	1	-24.968	4	0	1	0	1	0	1
193	2	max	1108.576	1	0	1	.656	1	0	1	0	1	0	1
194		min	-127.083	3	0	1	-25.024	4	0	1	-.002	4	0	1
195	3	max	1108.64	1	0	1	.656	1	0	1	0	1	0	1
196		min	-127.035	3	0	1	-25.08	4	0	1	-.004	4	0	1
197	4	max	1108.705	1	0	1	.656	1	0	1	0	1	0	1
198		min	-126.986	3	0	1	-25.136	4	0	1	-.007	4	0	1
199	5	max	1108.77	1	0	1	.656	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-126.938	3	0	1	-25.192	4	0	1	-.009	4	0	1
201		6	max	1108.834	1	0	1	.656	1	0	1	0	1	0	1
202			min	-126.889	3	0	1	-25.248	4	0	1	-.011	4	0	1
203		7	max	1108.899	1	0	1	.656	1	0	1	0	1	0	1
204			min	-126.841	3	0	1	-25.305	4	0	1	-.013	4	0	1
205		8	max	1108.964	1	0	1	.656	1	0	1	0	1	0	1
206			min	-126.792	3	0	1	-25.361	4	0	1	-.016	4	0	1
207		9	max	1109.028	1	0	1	.656	1	0	1	0	1	0	1
208			min	-126.743	3	0	1	-25.417	4	0	1	-.018	4	0	1
209		10	max	1109.093	1	0	1	.656	1	0	1	0	1	0	1
210			min	-126.695	3	0	1	-25.473	4	0	1	-.02	4	0	1
211		11	max	1109.158	1	0	1	.656	1	0	1	0	1	0	1
212			min	-126.646	3	0	1	-25.529	4	0	1	-.023	4	0	1
213		12	max	1109.223	1	0	1	.656	1	0	1	0	1	0	1
214			min	-126.598	3	0	1	-25.585	4	0	1	-.025	4	0	1
215		13	max	1109.287	1	0	1	.656	1	0	1	0	1	0	1
216			min	-126.549	3	0	1	-25.641	4	0	1	-.027	4	0	1
217		14	max	1109.352	1	0	1	.656	1	0	1	0	1	0	1
218			min	-126.501	3	0	1	-25.697	4	0	1	-.029	4	0	1
219		15	max	1109.417	1	0	1	.656	1	0	1	0	1	0	1
220			min	-126.452	3	0	1	-25.753	4	0	1	-.032	4	0	1
221		16	max	1109.481	1	0	1	.656	1	0	1	0	1	0	1
222			min	-126.404	3	0	1	-25.809	4	0	1	-.034	4	0	1
223		17	max	1109.546	1	0	1	.656	1	0	1	0	1	0	1
224			min	-126.355	3	0	1	-25.865	4	0	1	-.036	4	0	1
225		18	max	1109.611	1	0	1	.656	1	0	1	0	1	0	1
226			min	-126.307	3	0	1	-25.921	4	0	1	-.039	4	0	1
227		19	max	1109.676	1	0	1	.656	1	0	1	.001	1	0	1
228			min	-126.258	3	0	1	-25.977	4	0	1	-.041	4	0	1
229	M10	1	max	284.46	1	.668	4	1.256	5	0	1	0	1	0	1
230			min	-333.864	3	.168	15	-.109	1	-.002	5	0	3	0	1
231		2	max	284.576	1	.622	4	1.151	5	0	1	0	4	0	15
232			min	-333.777	3	.158	15	-.109	1	-.002	5	0	3	0	4
233		3	max	284.693	1	.576	4	1.045	5	0	1	0	4	0	15
234			min	-333.689	3	.147	15	-.109	1	-.002	5	0	3	0	4
235		4	max	284.809	1	.531	4	.94	5	0	1	0	4	0	15
236			min	-333.602	3	.136	15	-.109	1	-.002	5	0	3	0	4
237		5	max	284.925	1	.485	4	.834	5	0	1	0	4	0	15
238			min	-333.515	3	.125	15	-.109	1	-.002	5	0	3	0	4
239		6	max	285.042	1	.439	4	.729	5	0	1	0	4	0	15
240			min	-333.427	3	.115	15	-.109	1	-.002	5	0	3	0	4
241		7	max	285.158	1	.394	4	.623	5	0	1	0	4	0	15
242			min	-333.34	3	.104	15	-.109	1	-.002	5	0	3	0	4
243		8	max	285.275	1	.348	4	.518	5	0	1	0	4	0	15
244			min	-333.253	3	.093	15	-.109	1	-.002	5	0	3	0	4
245		9	max	285.391	1	.302	4	.413	5	0	1	.001	4	0	15
246			min	-333.165	3	.082	15	-.109	1	-.002	5	0	3	0	4
247		10	max	285.507	1	.257	4	.307	5	0	1	.001	4	0	15
248			min	-333.078	3	.072	15	-.109	1	-.002	5	0	3	0	4
249		11	max	285.624	1	.211	4	.202	5	0	1	.001	4	0	15
250			min	-332.991	3	.061	15	-.109	1	-.002	5	0	3	0	4
251		12	max	285.74	1	.165	4	.096	5	0	1	.001	4	0	15
252			min	-332.904	3	.05	15	-.109	1	-.002	5	0	3	0	4
253		13	max	285.857	1	.12	4	-.007	15	0	1	.001	4	0	15
254			min	-332.816	3	.037	12	-.109	1	-.002	5	0	3	0	4
255		14	max	285.973	1	.074	4	-.014	12	0	1	.001	5	0	15
256			min	-332.729	3	.006	1	-.132	4	-.002	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	286.089	1	.03	2	-.014	12	0	1	.001	5	0	15
258			min	-332.642	3	-.03	1	-.238	4	-.002	5	0	3	0	4
259		16	max	286.206	1	.01	5	-.014	12	0	1	.001	5	0	15
260			min	-332.554	3	-.065	1	-.343	4	-.002	5	0	3	0	4
261		17	max	286.322	1	-.003	15	-.014	12	0	1	.001	5	0	15
262			min	-332.467	3	-.101	1	-.449	4	-.002	5	0	3	0	4
263		18	max	286.439	1	-.014	15	-.014	12	0	1	0	5	0	15
264			min	-332.38	3	-.136	1	-.554	4	-.002	5	0	3	0	4
265		19	max	286.555	1	-.025	15	-.014	12	0	1	0	5	0	15
266			min	-332.292	3	-.172	1	-.66	4	-.002	5	0	3	0	4
267	M11	1	max	117.771	2	1.769	6	.489	1	.001	4	.001	5	0	6
268			min	-128.217	3	.412	15	-1.211	5	0	10	-.001	1	0	15
269		2	max	117.702	2	1.592	6	.489	1	.001	4	0	5	0	2
270			min	-128.268	3	.371	15	-1.078	5	0	10	-.001	1	0	12
271		3	max	117.634	2	1.415	6	.489	1	.001	4	0	5	0	2
272			min	-128.32	3	.329	15	-.944	5	0	10	-.001	1	0	3
273		4	max	117.565	2	1.238	6	.489	1	.001	4	0	5	0	15
274			min	-128.371	3	.287	15	-.811	5	0	10	-.001	1	0	4
275		5	max	117.496	2	1.06	6	.489	1	.001	4	0	3	0	15
276			min	-128.423	3	.246	15	-.677	5	0	10	-.001	1	0	4
277		6	max	117.428	2	.883	6	.489	1	.001	4	0	3	0	15
278			min	-128.474	3	.204	15	-.543	5	0	10	0	1	0	4
279		7	max	117.359	2	.706	6	.489	1	.001	4	0	3	0	15
280			min	-128.526	3	.162	15	-.41	5	0	10	0	1	0	4
281		8	max	117.291	2	.529	6	.489	1	.001	4	0	3	0	15
282			min	-128.577	3	.121	15	-.276	5	0	10	0	1	-.001	4
283		9	max	117.222	2	.352	6	.489	1	.001	4	0	3	0	15
284			min	-128.628	3	.079	15	-.142	5	0	10	0	1	-.001	4
285		10	max	117.153	2	.174	6	.489	1	.001	4	0	3	0	15
286			min	-128.68	3	.037	15	-.012	3	0	10	0	1	-.001	4
287		11	max	117.085	2	.026	2	.489	1	.001	4	0	3	0	15
288			min	-128.731	3	-.039	3	-.012	3	0	10	0	1	-.001	4
289		12	max	117.016	2	-.046	15	.489	1	.001	4	0	3	0	15
290			min	-128.783	3	-.18	4	-.012	3	0	10	0	1	-.001	4
291		13	max	116.947	2	-.088	15	.489	1	.001	4	0	3	0	15
292			min	-128.834	3	-.358	4	-.012	3	0	10	0	1	-.001	4
293		14	max	116.879	2	-.129	15	.621	4	.001	4	0	3	0	15
294			min	-128.886	3	-.535	4	-.012	3	0	10	0	1	-.001	4
295		15	max	116.81	2	-.171	15	.755	4	.001	4	0	3	0	15
296			min	-128.937	3	-.712	4	-.012	3	0	10	0	10	0	4
297		16	max	116.742	2	-.213	15	.888	4	.001	4	0	4	0	15
298			min	-128.989	3	-.889	4	-.012	3	0	10	0	10	0	4
299		17	max	116.673	2	-.254	15	1.022	4	.001	4	0	4	0	15
300			min	-129.04	3	-1.066	4	-.012	3	0	10	0	10	0	4
301		18	max	116.604	2	-.296	15	1.156	4	.001	4	0	4	0	15
302			min	-129.092	3	-1.244	4	-.012	3	0	10	0	10	0	4
303		19	max	116.536	2	-.337	15	1.289	4	.001	4	.001	4	0	1
304			min	-129.143	3	-1.421	4	-.012	3	0	10	0	10	0	1
305	M12	1	max	399.223	1	0	1	2.396	1	0	1	0	4	0	1
306			min	-29.914	3	0	1	-22.857	5	0	1	0	3	0	1
307		2	max	399.288	1	0	1	2.396	1	0	1	0	1	0	1
308			min	-29.866	3	0	1	-22.913	5	0	1	-.002	5	0	1
309		3	max	399.352	1	0	1	2.396	1	0	1	0	1	0	1
310			min	-29.817	3	0	1	-22.969	5	0	1	-.004	5	0	1
311		4	max	399.417	1	0	1	2.396	1	0	1	0	1	0	1
312			min	-29.769	3	0	1	-23.025	5	0	1	-.006	5	0	1
313		5	max	399.482	1	0	1	2.396	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	-29.72	3	0	1	-23.081	5	0	1	-.008	5	0	1
315		6	max	399.546	1	0	1	2.396	1	0	1	.001	1	0	1
316			min	-29.672	3	0	1	-23.137	5	0	1	-.01	5	0	1
317		7	max	399.611	1	0	1	2.396	1	0	1	.001	1	0	1
318			min	-29.623	3	0	1	-23.193	5	0	1	-.012	5	0	1
319		8	max	399.676	1	0	1	2.396	1	0	1	.002	1	0	1
320			min	-29.575	3	0	1	-23.249	5	0	1	-.014	5	0	1
321		9	max	399.741	1	0	1	2.396	1	0	1	.002	1	0	1
322			min	-29.526	3	0	1	-23.305	5	0	1	-.016	5	0	1
323		10	max	399.805	1	0	1	2.396	1	0	1	.002	1	0	1
324			min	-29.478	3	0	1	-23.361	5	0	1	-.019	5	0	1
325		11	max	399.87	1	0	1	2.396	1	0	1	.002	1	0	1
326			min	-29.429	3	0	1	-23.418	5	0	1	-.021	5	0	1
327		12	max	399.935	1	0	1	2.396	1	0	1	.002	1	0	1
328			min	-29.381	3	0	1	-23.474	5	0	1	-.023	5	0	1
329		13	max	399.999	1	0	1	2.396	1	0	1	.003	1	0	1
330			min	-29.332	3	0	1	-23.53	5	0	1	-.025	5	0	1
331		14	max	400.064	1	0	1	2.396	1	0	1	.003	1	0	1
332			min	-29.284	3	0	1	-23.586	5	0	1	-.027	5	0	1
333		15	max	400.129	1	0	1	2.396	1	0	1	.003	1	0	1
334			min	-29.235	3	0	1	-23.642	5	0	1	-.029	5	0	1
335		16	max	400.194	1	0	1	2.396	1	0	1	.003	1	0	1
336			min	-29.186	3	0	1	-23.698	5	0	1	-.031	5	0	1
337		17	max	400.258	1	0	1	2.396	1	0	1	.003	1	0	1
338			min	-29.138	3	0	1	-23.754	5	0	1	-.033	5	0	1
339		18	max	400.323	1	0	1	2.396	1	0	1	.004	1	0	1
340			min	-29.089	3	0	1	-23.81	5	0	1	-.035	5	0	1
341		19	max	400.388	1	0	1	2.396	1	0	1	.004	1	0	1
342			min	-29.041	3	0	1	-23.866	5	0	1	-.038	5	0	1
343	M1	1	max	112.534	1	339.316	3	-2.715	12	0	1	.093	1	0	1
344			min	5.267	12	-281.556	1	-47.479	1	0	3	.006	12	0	3
345		2	max	112.652	1	339.126	3	-2.715	12	0	1	.083	1	.061	1
346			min	5.326	12	-281.809	1	-47.479	1	0	3	.006	12	-.074	3
347		3	max	74.047	1	5.938	9	-2.758	12	0	12	.072	1	.121	1
348			min	-5.344	10	-18.516	2	-47.284	1	0	1	.005	12	-.146	3
349		4	max	74.165	1	5.727	9	-2.758	12	0	12	.062	1	.122	1
350			min	-5.246	10	-18.77	2	-47.284	1	0	1	.004	12	-.142	3
351		5	max	74.283	1	5.516	9	-2.758	12	0	12	.051	1	.123	1
352			min	-5.147	10	-19.023	2	-47.284	1	0	1	.004	12	-.138	3
353		6	max	74.401	1	5.305	9	-2.758	12	0	12	.041	1	.123	1
354			min	-5.049	10	-19.276	2	-47.284	1	0	1	.003	12	-.134	3
355		7	max	74.519	1	5.094	9	-2.758	12	0	12	.031	1	.125	2
356			min	-4.951	10	-19.529	2	-47.284	1	0	1	.002	10	-.13	3
357		8	max	74.637	1	4.883	9	-2.758	12	0	12	.021	1	.129	2
358			min	-4.852	10	-19.782	2	-47.284	1	0	1	.002	10	-.126	3
359		9	max	74.755	1	4.672	9	-2.758	12	0	12	.01	1	.134	2
360			min	-4.754	10	-20.035	2	-47.284	1	0	1	0	10	-.122	3
361		10	max	74.873	1	4.461	9	-2.758	12	0	12	.002	4	.138	2
362			min	-4.656	10	-20.288	2	-47.284	1	0	1	0	10	-.118	3
363		11	max	74.991	1	4.25	9	-2.758	12	0	12	0	3	.143	2
364			min	-4.557	10	-20.541	2	-47.284	1	0	1	-.01	1	-.114	3
365		12	max	75.109	1	4.04	9	-2.758	12	0	12	0	12	.147	2
366			min	-4.459	10	-20.794	2	-47.284	1	0	1	-.02	1	-.109	3
367		13	max	75.227	1	3.829	9	-2.758	12	0	12	-.001	12	.152	2
368			min	-4.361	10	-21.047	2	-47.284	1	0	1	-.031	1	-.105	3
369		14	max	75.345	1	3.618	9	-2.758	12	0	12	-.002	12	.156	2
370			min	-4.262	10	-21.3	2	-47.284	1	0	1	-.041	1	-.101	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	75.463	1	3.407	9	-2.758	12	0	12	-.002	12	.161	2
372			min	-4.164	10	-21.553	2	-47.284	1	0	1	-.051	1	-.096	3
373		16	max	86.479	2	69.2	2	-2.789	12	0	1	-.003	12	.165	2
374			min	-19.712	3	-121.3	3	-47.633	1	0	5	-.062	1	-.091	3
375		17	max	86.597	2	68.947	2	-2.789	12	0	1	-.004	12	.15	2
376			min	-19.624	3	-121.49	3	-47.633	1	0	5	-.072	1	-.065	3
377		18	max	-4.444	12	347.116	2	-2.95	12	0	3	-.004	12	.076	2
378			min	-112.613	1	-149.252	3	-48.749	1	0	2	-.083	1	-.033	3
379		19	max	-4.385	12	346.862	2	-2.95	12	0	3	-.005	12	0	2
380			min	-112.495	1	-149.442	3	-48.749	1	0	2	-.093	1	0	3
381	M5	1	max	252.742	1	1116.966	3	0	10	0	1	.038	4	0	3
382			min	5.874	15	-926.705	1	-45.11	3	0	5	0	10	0	1
383		2	max	252.86	1	1116.776	3	0	10	0	1	.032	4	.201	1
384			min	5.91	15	-926.958	1	-45.11	3	0	5	-.004	3	-.242	3
385		3	max	175.71	3	6.405	9	5.124	3	0	3	.027	4	.398	1
386			min	-25.405	10	-70.219	2	-20.608	4	0	4	-.014	3	-.479	3
387		4	max	175.798	3	6.194	9	5.124	3	0	3	.023	4	.404	1
388			min	-25.307	10	-70.472	2	-20.366	4	0	4	-.013	3	-.465	3
389		5	max	175.887	3	5.983	9	5.124	3	0	3	.018	4	.41	1
390			min	-25.208	10	-70.725	2	-20.124	4	0	4	-.012	3	-.451	3
391		6	max	175.975	3	5.772	9	5.124	3	0	3	.014	4	.416	1
392			min	-25.11	10	-70.979	2	-19.882	4	0	4	-.01	3	-.437	3
393		7	max	176.064	3	5.561	9	5.124	3	0	3	.01	4	.422	1
394			min	-25.012	10	-71.232	2	-19.64	4	0	4	-.009	3	-.423	3
395		8	max	176.152	3	5.35	9	5.124	3	0	3	.005	4	.432	2
396			min	-24.913	10	-71.485	2	-19.398	4	0	4	-.008	3	-.409	3
397		9	max	176.241	3	5.139	9	5.124	3	0	3	.001	5	.448	2
398			min	-24.815	10	-71.738	2	-19.156	4	0	4	-.007	3	-.394	3
399		10	max	176.329	3	4.929	9	5.124	3	0	3	0	10	.463	2
400			min	-24.717	10	-71.991	2	-18.914	4	0	4	-.006	3	-.38	3
401		11	max	176.418	3	4.718	9	5.124	3	0	3	0	10	.479	2
402			min	-24.618	10	-72.244	2	-18.672	4	0	4	-.007	4	-.366	3
403		12	max	176.506	3	4.507	9	5.124	3	0	3	0	10	.495	2
404			min	-24.52	10	-72.497	2	-18.43	4	0	4	-.011	4	-.352	3
405		13	max	176.595	3	4.296	9	5.124	3	0	3	0	10	.51	2
406			min	-24.422	10	-72.75	2	-18.188	4	0	4	-.015	4	-.337	3
407		14	max	176.683	3	4.085	9	5.124	3	0	3	0	10	.526	2
408			min	-24.323	10	-73.003	2	-17.946	4	0	4	-.019	4	-.323	3
409		15	max	176.772	3	3.874	9	5.124	3	0	3	0	10	.542	2
410			min	-24.225	10	-73.256	2	-17.704	4	0	4	-.023	4	-.308	3
411		16	max	293.314	2	298.931	2	5.093	3	0	1	0	3	.555	2
412			min	-65.354	3	-371.269	3	-16.429	4	0	4	-.027	4	-.291	3
413		17	max	293.432	2	298.678	2	5.093	3	0	1	.001	3	.49	2
414			min	-65.265	3	-371.459	3	-16.187	4	0	4	-.03	4	-.211	3
415		18	max	-7.801	12	1138.542	2	4.669	3	0	4	.002	3	.246	2
416			min	-252.907	1	-487.355	3	-40.171	5	0	1	-.039	4	-.105	3
417		19	max	-7.742	12	1138.289	2	4.669	3	0	4	.003	3	0	3
418			min	-252.789	1	-487.545	3	-39.929	5	0	1	-.047	4	0	2
419	M9	1	max	112.091	1	339.282	3	167.525	4	0	3	0	15	0	1
420			min	2.105	15	-281.554	1	3.64	10	0	1	-.092	1	0	3
421		2	max	112.209	1	339.092	3	167.767	4	0	3	.033	5	.061	1
422			min	2.14	15	-281.807	1	3.64	10	0	1	-.081	1	-.074	3
423		3	max	74.088	1	5.913	9	46.048	1	0	1	.065	5	.121	1
424			min	-4.902	10	-18.528	2	-26.83	5	0	5	-.07	1	-.146	3
425		4	max	74.206	1	5.703	9	46.048	1	0	1	.059	5	.122	1
426			min	-4.803	10	-18.781	2	-26.588	5	0	5	-.06	1	-.142	3
427		5	max	74.324	1	5.492	9	46.048	1	0	1	.054	5	.123	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-4.705	10	-19.034	2	-26.346	5	0	5	-.05	1	-.138	3
429	6	max	74.442	1	5.281	9	46.048	1	0	1	.048	5	.123	1
430		min	-4.607	10	-19.287	2	-26.104	5	0	5	-.04	1	-.134	3
431	7	max	74.56	1	5.07	9	46.048	1	0	1	.042	5	.125	2
432		min	-4.508	10	-19.54	2	-25.862	5	0	5	-.03	1	-.13	3
433	8	max	74.678	1	4.859	9	46.048	1	0	1	.037	5	.129	2
434		min	-4.41	10	-19.793	2	-25.62	5	0	5	-.02	1	-.126	3
435	9	max	74.796	1	4.648	9	46.048	1	0	1	.031	5	.134	2
436		min	-4.312	10	-20.046	2	-25.378	5	0	5	-.01	1	-.122	3
437	10	max	74.914	1	4.437	9	46.048	1	0	1	.026	4	.138	2
438		min	-4.213	10	-20.299	2	-25.136	5	0	5	0	2	-.118	3
439	11	max	75.032	1	4.226	9	46.048	1	0	1	.022	4	.142	2
440		min	-4.115	10	-20.552	2	-24.894	5	0	5	0	10	-.114	3
441	12	max	75.15	1	4.015	9	46.048	1	0	1	.02	1	.147	2
442		min	-4.017	10	-20.806	2	-24.652	5	0	5	.002	10	-.109	3
443	13	max	75.268	1	3.804	9	46.048	1	0	1	.03	1	.151	2
444		min	-3.918	10	-21.059	2	-24.41	5	0	5	.002	10	-.105	3
445	14	max	75.386	1	3.594	9	46.048	1	0	1	.04	1	.156	2
446		min	-3.82	10	-21.312	2	-24.168	5	0	5	.003	15	-.101	3
447	15	max	75.504	1	3.383	9	46.048	1	0	1	.05	1	.161	2
448		min	-3.722	10	-21.565	2	-23.926	5	0	5	0	5	-.096	3
449	16	max	86.697	2	68.926	2	46.492	1	0	10	.061	1	.165	2
450		min	-20.009	3	-121.722	3	-22.494	5	0	4	-.005	5	-.091	3
451	17	max	86.815	2	68.673	2	46.492	1	0	10	.071	1	.15	2
452		min	-19.92	3	-121.912	3	-22.252	5	0	4	-.009	5	-.065	3
453	18	max	3.559	5	347.116	2	48.892	1	0	2	.081	1	.076	2
454		min	-112.201	1	-149.248	3	-44.823	5	0	3	-.019	5	-.033	3
455	19	max	3.614	5	346.863	2	48.892	1	0	2	.092	1	0	2
456		min	-112.083	1	-149.438	3	-44.581	5	0	3	-.029	5	0	3
457	M13	1	max	167.53	4	281.202	1	-2.105	15	0	.092	1	0	1
458		min	3.641	10	-339.289	3	-112.081	1	0	3	0	15	0	3
459	2	max	160.982	4	198.641	1	-1.141	15	0	1	.026	1	.193	3
460		min	3.641	10	-239.545	3	-85.561	1	0	3	0	5	-.16	1
461	3	max	154.434	4	116.08	1	-.178	15	0	1	.004	3	.319	3
462		min	3.641	10	-139.801	3	-59.04	1	0	3	-.022	1	-.265	1
463	4	max	147.886	4	33.52	1	1.099	5	0	1	.002	3	.379	3
464		min	3.641	10	-40.057	3	-32.52	1	0	3	-.053	1	-.315	1
465	5	max	141.339	4	59.687	3	2.589	5	0	1	0	15	.373	3
466		min	3.641	10	-49.041	1	-5.999	1	0	3	-.065	1	-.31	1
467	6	max	134.791	4	159.431	3	20.521	1	0	1	.002	5	.3	3
468		min	3.641	10	-131.602	1	-.663	3	0	3	-.061	1	-.249	1
469	7	max	128.243	4	259.175	3	47.042	1	0	1	.005	5	.16	3
470		min	3.641	10	-214.163	1	.564	12	0	3	-.038	1	-.134	1
471	8	max	121.695	4	358.919	3	73.562	1	0	1	.01	4	.036	1
472		min	3.641	10	-296.724	1	1.498	12	0	3	0	3	-.046	3
473	9	max	115.148	4	458.663	3	100.083	1	0	1	.06	1	.262	1
474		min	3.641	10	-379.284	1	2.433	12	0	3	.001	12	-.318	3
475	10	max	108.6	4	558.407	3	126.603	1	0	1	.136	1	.542	1
476		min	3.641	10	-461.845	1	3.367	12	0	3	.003	12	-.657	3
477	11	max	78.303	4	379.284	1	2.243	5	0	3	.059	1	.262	1
478		min	2.716	12	-458.663	3	-99.639	1	0	1	-.015	5	-.318	3
479	12	max	71.755	4	296.724	1	3.733	5	0	3	.003	2	.036	1
480		min	2.716	12	-358.919	3	-73.119	1	0	1	-.013	4	-.046	3
481	13	max	65.207	4	214.163	1	5.223	5	0	3	-.003	10	.16	3
482		min	2.716	12	-259.175	3	-46.599	1	0	1	-.039	1	-.134	1
483	14	max	58.66	4	131.602	1	6.713	5	0	3	-.004	12	.3	3
484		min	2.716	12	-159.431	3	-20.078	1	0	1	-.061	1	-.249	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

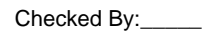
Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	52.112	4	49.041	1	9.771	4	0	3	0	15	.373	3
486			min	2.716	12	-59.687	3	-.133	10	0	1	-.065	1	-.31	1
487		16	max	47.604	1	40.057	3	32.963	1	0	3	.005	5	.379	3
488			min	2.716	12	-33.52	1	2.464	12	0	1	-.052	1	-.315	1
489		17	max	47.604	1	139.801	3	59.483	1	0	3	.012	5	.319	3
490			min	2.716	12	-116.081	1	3.399	12	0	1	-.021	1	-.265	1
491		18	max	47.604	1	239.545	3	86.004	1	0	3	.027	1	.193	3
492			min	2.716	12	-198.641	1	4.333	12	0	1	.001	10	-.16	1
493		19	max	47.604	1	339.289	3	112.524	1	0	3	.093	1	0	1
494			min	2.716	12	-281.202	1	5.268	12	0	1	.006	12	0	3
495	M16	1	max	44.572	5	347.028	2	3.614	5	0	3	.092	1	0	2
496			min	-48.759	1	-149.458	3	-112.094	1	0	2	-.029	5	0	3
497		2	max	38.024	5	245.15	2	5.104	5	0	3	.026	1	.085	3
498			min	-48.759	1	-105.76	3	-85.574	1	0	2	-.026	5	-.197	2
499		3	max	31.477	5	143.273	2	6.594	5	0	3	0	12	.141	3
500			min	-48.759	1	-62.062	3	-59.053	1	0	2	-.026	4	-.327	2
501		4	max	24.929	5	41.395	2	8.084	5	0	3	-.002	12	.168	3
502			min	-48.759	1	-18.363	3	-32.533	1	0	2	-.053	1	-.388	2
503		5	max	18.381	5	25.335	3	9.574	5	0	3	-.003	12	.165	3
504			min	-48.759	1	-60.483	2	-6.012	1	0	2	-.066	1	-.382	2
505		6	max	11.833	5	69.033	3	20.508	1	0	3	-.003	15	.134	3
506			min	-48.759	1	-162.36	2	-.103	3	0	2	-.061	1	-.308	2
507		7	max	5.286	5	112.731	3	47.029	1	0	3	.004	5	.073	3
508			min	-48.759	1	-264.238	2	.914	12	0	2	-.038	1	-.166	2
509		8	max	.129	3	156.429	3	73.549	1	0	3	.013	4	.045	2
510			min	-48.759	1	-366.115	2	1.848	12	0	2	-.003	3	-.016	3
511		9	max	.129	3	200.127	3	100.07	1	0	3	.06	1	.323	2
512			min	-48.759	1	-467.993	2	2.783	12	0	2	-.001	3	-.135	3
513		10	max	25.59	5	-11.55	15	126.59	1	0	14	.135	1	.669	2
514			min	-48.759	1	-569.871	2	-5.999	3	0	2	.004	12	-.283	3
515		11	max	19.042	5	467.993	2	2.041	5	0	2	.059	1	.323	2
516			min	-48.626	1	-200.127	3	-99.658	1	0	3	-.013	5	-.135	3
517		12	max	12.494	5	366.115	2	3.531	5	0	2	.003	2	.045	2
518			min	-48.626	1	-156.429	3	-73.137	1	0	3	-.011	4	-.016	3
519		13	max	5.946	5	264.238	2	5.021	5	0	2	-.001	12	.073	3
520			min	-48.626	1	-112.731	3	-46.617	1	0	3	-.038	1	-.166	2
521		14	max	-.333	15	162.36	2	6.511	5	0	2	-.002	12	.134	3
522			min	-48.626	1	-69.033	3	-20.096	1	0	3	-.061	1	-.308	2
523		15	max	-2.95	12	60.482	2	9.545	4	0	2	0	5	.165	3
524			min	-48.626	1	-25.335	3	-.126	10	0	3	-.065	1	-.382	2
525		16	max	-2.95	12	18.363	3	32.945	1	0	2	.007	5	.168	3
526			min	-48.626	1	-41.395	2	1.581	12	0	3	-.052	1	-.388	2
527		17	max	-2.95	12	62.062	3	59.465	1	0	2	.013	5	.141	3
528			min	-48.626	1	-143.273	2	2.516	12	0	3	-.021	1	-.327	2
529		18	max	-2.95	12	105.76	3	85.985	1	0	2	.027	4	.085	3
530			min	-48.626	1	-245.15	2	3.45	12	0	3	.001	10	-.197	2
531		19	max	-2.95	12	149.458	3	112.506	1	0	2	.093	1	0	2
532			min	-48.626	1	-347.028	2	4.385	12	0	3	.005	12	0	3
533	M15	1	max	.228	1	1.638	1	.059	3	0	1	0	1	0	1
534			min	-56.005	3	0	2	-.056	1	0	3	0	3	0	1
535		2	max	.141	1	1.456	1	.059	3	0	1	0	1	0	2
536			min	-56.07	3	0	2	-.056	1	0	3	0	3	0	1
537		3	max	.054	1	1.274	1	.059	3	0	1	0	1	0	2
538			min	-56.135	3	0	2	-.056	1	0	3	0	3	-.001	1
539		4	max	0	2	1.092	1	.059	3	0	1	0	1	0	2
540			min	-56.201	3	0	2	-.056	1	0	3	0	3	-.002	1
541		5	max	0	2	.91	1	.059	3	0	1	0	1	0	2





RISA-3D Version 13.0.0 \\\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.\\.PVMMini 60 Cell 1V 25° 100mph 30psf 6ft 7-05.r3dPage 32



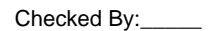
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.009	5	1.237e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-8.725e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.013	5	1.434e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-8.807e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.018	5	1.631e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-8.889e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.022	4	1.828e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-8.97e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.027	4	2.025e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	0	1	-9.052e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.031	4	2.222e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	-9.134e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.035	4	2.419e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	1	-9.216e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.04	4	2.616e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-9.298e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.044	4	2.813e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-9.38e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.048	4	3.01e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-9.461e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.052	4	3.207e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-9.543e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.056	4	3.404e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-9.625e-4	5	NC	1	NC	1
67		15	max	.001	3	.005	2	.06	4	3.601e-4	1	NC	1	NC	1
68			min	-.001	2	-.007	3	0	10	-9.707e-4	5	9658.459	2	NC	1
69		16	max	.001	3	.006	2	.064	4	3.798e-4	1	NC	1	NC	1
70			min	-.001	2	-.008	3	0	10	-9.789e-4	5	8138.898	2	NC	1
71		17	max	.001	3	.007	2	.067	4	3.995e-4	1	NC	1	NC	1
72			min	-.001	2	-.008	3	0	10	-9.87e-4	5	6972.478	2	NC	1
73		18	max	.001	3	.008	2	.071	4	4.192e-4	1	NC	3	NC	1
74			min	-.001	2	-.008	3	0	10	-9.952e-4	5	6065.967	2	NC	1
75		19	max	.001	3	.009	2	.075	4	4.389e-4	1	NC	3	NC	1
76			min	-.001	2	-.008	3	0	10	-1.003e-3	5	5354.565	2	NC	1
77	M4	1	max	.002	1	.009	2	0	10	4.525e-3	5	NC	1	NC	2
78			min	0	3	-.008	3	-.079	4	-5.943e-4	1	NC	1	243.82	4
79		2	max	.002	1	.009	2	0	10	4.525e-3	5	NC	1	NC	2
80			min	0	3	-.007	3	-.073	4	-5.943e-4	1	NC	1	265.79	4
81		3	max	.002	1	.008	2	0	10	4.525e-3	5	NC	1	NC	2
82			min	0	3	-.007	3	-.066	4	-5.943e-4	1	NC	1	291.939	4
83		4	max	.002	1	.008	2	0	10	4.525e-3	5	NC	1	NC	2
84			min	0	3	-.006	3	-.06	4	-5.943e-4	1	NC	1	323.368	4
85		5	max	.001	1	.007	2	0	10	4.525e-3	5	NC	1	NC	2
86			min	0	3	-.006	3	-.053	4	-5.943e-4	1	NC	1	361.575	4
87		6	max	.001	1	.007	2	0	10	4.525e-3	5	NC	1	NC	2
88			min	0	3	-.005	3	-.047	4	-5.943e-4	1	NC	1	408.646	4
89		7	max	.001	1	.006	2	0	10	4.525e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.041	4	-5.943e-4	1	NC	1	467.551	4
91		8	max	.001	1	.006	2	0	10	4.525e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.036	4	-5.943e-4	1	NC	1	542.637	4
93		9	max	.001	1	.005	2	0	10	4.525e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.03	4	-5.943e-4	1	NC	1	640.482	4
95		10	max	0	1	.005	2	0	10	4.525e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.025	4	-5.943e-4	1	NC	1	771.423	4
97		11	max	0	1	.004	2	0	10	4.525e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.02	4	-5.943e-4	1	NC	1	952.488	4
99		12	max	0	1	.004	2	0	10	4.525e-3	5	NC	1	NC	1





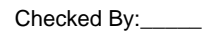
Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.009	4	2.405e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-8.824e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.014	4	1.823e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-8.767e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.019	4	1.241e-5	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-8.71e-4	4	8468.963	2	NC	1
163		6	max	.001	3	.007	2	.023	4	1.781e-5	3	NC	3	NC	1
164			min	-.001	2	-.009	3	0	1	-8.653e-4	4	6791.746	2	NC	1
165		7	max	.001	3	.008	2	.028	4	3.484e-5	3	NC	3	NC	1
166			min	-.002	2	-.01	3	0	1	-8.596e-4	4	5645.15	2	NC	1
167		8	max	.002	3	.01	2	.032	4	5.186e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	-.001	1	-8.538e-4	4	4804.763	2	NC	1
169		9	max	.002	3	.011	2	.037	4	6.889e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	-.001	1	-8.481e-4	4	4158.781	2	NC	1
171		10	max	.002	3	.013	2	.041	4	8.592e-5	3	NC	3	NC	1
172			min	-.003	2	-.015	3	-.001	1	-8.424e-4	4	3645.247	2	NC	1
173		11	max	.002	3	.014	2	.045	4	1.029e-4	3	NC	3	NC	1
174			min	-.003	2	-.016	3	-.002	1	-8.367e-4	4	3226.973	2	NC	1
175		12	max	.003	3	.016	2	.049	4	1.2e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	-.002	1	-8.31e-4	4	2880.173	2	NC	1
177		13	max	.003	3	.018	2	.053	4	1.37e-4	3	NC	3	NC	1
178			min	-.004	2	-.018	3	-.002	1	-8.253e-4	4	2588.815	2	NC	1
179		14	max	.003	3	.02	2	.057	4	1.54e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	-.002	1	-8.196e-4	4	2341.604	2	NC	1
181		15	max	.003	3	.022	2	.061	4	1.711e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	-.002	1	-8.138e-4	4	2130.289	2	NC	1
183		16	max	.004	3	.024	2	.065	4	1.881e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	-.002	1	-8.081e-4	4	1948.647	2	NC	1
185		17	max	.004	3	.026	2	.069	4	2.051e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	-.002	1	-8.024e-4	4	1791.875	2	NC	1
187		18	max	.004	3	.028	2	.072	4	2.221e-4	3	NC	3	NC	1
188			min	-.005	2	-.023	3	-.002	1	-7.967e-4	4	1656.185	2	NC	1
189		19	max	.004	3	.03	2	.076	4	2.392e-4	3	NC	3	NC	1
190			min	-.005	2	-.024	3	-.002	1	-7.91e-4	4	1538.553	2	NC	1
191	M8	1	max	.005	1	.032	2	.002	1	4.331e-3	4	NC	1	NC	2
192			min	0	3	-.024	3	-.08	4	-1.845e-4	3	NC	1	241.655	4
193		2	max	.005	1	.03	2	.002	1	4.331e-3	4	NC	1	NC	1
194			min	0	3	-.023	3	-.073	4	-1.845e-4	3	NC	1	263.43	4
195		3	max	.005	1	.028	2	.002	1	4.331e-3	4	NC	1	NC	1
196			min	0	3	-.021	3	-.067	4	-1.845e-4	3	NC	1	289.346	4
197		4	max	.004	1	.026	2	.002	1	4.331e-3	4	NC	1	NC	1
198			min	0	3	-.02	3	-.06	4	-1.845e-4	3	NC	1	320.495	4
199		5	max	.004	1	.025	2	.001	1	4.331e-3	4	NC	1	NC	1
200			min	0	3	-.019	3	-.054	4	-1.845e-4	3	NC	1	358.362	4
201		6	max	.004	1	.023	2	.001	1	4.331e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.048	4	-1.845e-4	3	NC	1	405.015	4
203		7	max	.004	1	.021	2	.001	1	4.331e-3	4	NC	1	NC	1
204			min	0	3	-.016	3	-.042	4	-1.845e-4	3	NC	1	463.395	4
205		8	max	.003	1	.019	2	0	1	4.331e-3	4	NC	1	NC	1
206			min	0	3	-.015	3	-.036	4	-1.845e-4	3	NC	1	537.814	4
207		9	max	.003	1	.018	2	0	1	4.331e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.03	4	-1.845e-4	3	NC	1	634.789	4
209		10	max	.003	1	.016	2	0	1	4.331e-3	4	NC	1	NC	1
210			min	0	3	-.012	3	-.025	4	-1.845e-4	3	NC	1	764.566	4
211		11	max	.002	1	.014	2	0	1	4.331e-3	4	NC	1	NC	1
212			min	0	3	-.011	3	-.02	4	-1.845e-4	3	NC	1	944.022	4
213		12	max	.002	1	.012	2	0	1	4.331e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.007	4	3.497e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-8.4e-4	4	NC	1	6467.076	5
273		4	max	0	3	0	2	.011	4	1.737e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-9.167e-4	4	NC	1	4272.301	5
275		5	max	0	3	0	2	.015	4	-2.336e-7	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-9.933e-4	4	NC	1	3181.206	5
277		6	max	0	3	0	2	.018	5	-1.165e-5	12	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-1.07e-3	4	NC	1	2530.873	5
279		7	max	0	3	0	2	.022	5	-1.643e-5	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-1.147e-3	4	NC	1	2100.39	5
281		8	max	0	3	0	2	.026	5	-1.844e-5	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-1.223e-3	4	NC	1	1795.116	5
283		9	max	0	3	.001	2	.029	5	-2.044e-5	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-1.3e-3	4	NC	1	1567.757	5
285		10	max	0	3	.002	2	.033	5	-2.245e-5	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	1	-1.376e-3	4	NC	1	1392.05	5
287		11	max	0	3	.002	2	.037	5	-2.445e-5	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	1	-1.453e-3	4	NC	1	1252.248	5
289		12	max	0	3	.003	2	.04	5	-2.646e-5	10	NC	1	NC	1
290			min	0	2	-.007	3	-.004	1	-1.53e-3	4	NC	1	1138.332	5
291		13	max	0	3	.003	2	.044	5	-2.846e-5	10	NC	1	NC	2
292			min	0	2	-.007	3	-.005	1	-1.606e-3	4	NC	1	1043.625	5
293		14	max	.001	3	.004	2	.048	5	-3.047e-5	10	NC	1	NC	2
294			min	0	2	-.007	3	-.005	1	-1.683e-3	4	NC	1	963.505	5
295		15	max	.001	3	.005	2	.051	5	-3.247e-5	10	NC	1	NC	2
296			min	-.001	2	-.008	3	-.006	1	-1.76e-3	4	9674.019	2	894.671	5
297		16	max	.001	3	.006	2	.055	5	-3.448e-5	10	NC	1	NC	2
298			min	-.001	2	-.008	3	-.007	1	-1.836e-3	4	8150.681	2	834.7	5
299		17	max	.001	3	.007	2	.059	5	-3.648e-5	10	NC	1	NC	2
300			min	-.001	2	-.008	3	-.008	1	-1.913e-3	4	6981.663	2	781.779	5
301		18	max	.001	3	.008	2	.063	5	-3.849e-5	10	NC	3	NC	2
302			min	-.001	2	-.008	3	-.008	1	-1.99e-3	4	6073.323	2	734.521	5
303		19	max	.001	3	.009	2	.067	5	-4.049e-5	10	NC	3	NC	2
304			min	-.001	2	-.008	3	-.009	1	-2.066e-3	4	5360.612	2	691.853	5
305	M12	1	max	.002	1	.009	2	.008	1	5.446e-3	4	NC	1	NC	3
306			min	0	3	-.008	3	-.073	5	4.329e-5	10	NC	1	263.667	5
307		2	max	.002	1	.009	2	.007	1	5.446e-3	4	NC	1	NC	3
308			min	0	3	-.007	3	-.067	5	4.329e-5	10	NC	1	287.419	5
309		3	max	.002	1	.008	2	.006	1	5.446e-3	4	NC	1	NC	3
310			min	0	3	-.007	3	-.061	5	4.329e-5	10	NC	1	315.689	5
311		4	max	.002	1	.008	2	.006	1	5.446e-3	4	NC	1	NC	2
312			min	0	3	-.006	3	-.055	5	4.329e-5	10	NC	1	349.666	5
313		5	max	.001	1	.007	2	.005	1	5.446e-3	4	NC	1	NC	2
314			min	0	3	-.006	3	-.049	5	4.329e-5	10	NC	1	390.97	5
315		6	max	.001	1	.007	2	.005	1	5.446e-3	4	NC	1	NC	2
316			min	0	3	-.005	3	-.044	5	4.329e-5	10	NC	1	441.857	5
317		7	max	.001	1	.006	2	.004	1	5.446e-3	4	NC	1	NC	2
318			min	0	3	-.005	3	-.038	5	4.329e-5	10	NC	1	505.535	5
319		8	max	.001	1	.006	2	.003	1	5.446e-3	4	NC	1	NC	2
320			min	0	3	-.005	3	-.033	5	4.329e-5	10	NC	1	586.705	5
321		9	max	.001	1	.005	2	.003	1	5.446e-3	4	NC	1	NC	2
322			min	0	3	-.004	3	-.028	5	4.329e-5	10	NC	1	692.475	5
323		10	max	0	1	.005	2	.002	1	5.446e-3	4	NC	1	NC	2
324			min	0	3	-.004	3	-.023	5	4.329e-5	10	NC	1	834.022	5
325		11	max	0	1	.004	2	.002	1	5.446e-3	4	NC	1	NC	2
326			min	0	3	-.003	3	-.019	5	4.329e-5	10	NC	1	1029.748	5
327		12	max	0	1	.004	2	.002	1	5.446e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.003	3	-.015	5	4.329e-5	10	NC	1	1311.758	5
329		13	max	0	1	.003	2	.001	1	5.446e-3	4	NC	1	NC	1
330			min	0	3	-.003	3	-.011	5	4.329e-5	10	NC	1	1740.264	5
331		14	max	0	1	.003	2	0	1	5.446e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.008	5	4.329e-5	10	NC	1	2439.471	5
333		15	max	0	1	.002	2	0	1	5.446e-3	4	NC	1	NC	1
334			min	0	3	-.002	3	-.005	5	4.329e-5	10	NC	1	3701.167	5
335		16	max	0	1	.002	2	0	1	5.446e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.003	5	4.329e-5	10	NC	1	6354.83	5
337		17	max	0	1	.001	2	0	1	5.446e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.001	5	4.329e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	5.446e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	4.329e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	5.446e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	4.329e-5	10	NC	1	NC	1
343	M1	1	max	.007	3	.023	3	.007	5	1.424e-2	1	NC	1	NC	1
344			min	-.007	2	-.021	2	-.004	1	-1.701e-2	3	NC	1	NC	1
345		2	max	.007	3	.013	3	.01	5	6.825e-3	1	NC	4	NC	1
346			min	-.007	2	-.012	2	-.007	1	-8.413e-3	3	4915.433	2	NC	1
347		3	max	.007	3	.004	3	.014	5	4.273e-4	5	NC	4	NC	2
348			min	-.008	2	-.003	2	-.01	1	-4.514e-4	1	2525.093	2	7112.937	5
349		4	max	.007	3	.005	2	.018	5	4.315e-4	5	NC	4	NC	2
350			min	-.008	2	-.003	3	-.011	1	-3.845e-4	1	1769.402	2	4498.312	5
351		5	max	.007	3	.011	2	.022	5	4.356e-4	5	NC	4	NC	2
352			min	-.008	2	-.01	3	-.011	1	-3.176e-4	1	1404.101	2	3223.746	5
353		6	max	.007	3	.017	2	.026	5	4.398e-4	5	NC	5	NC	2
354			min	-.008	2	-.015	3	-.01	1	-2.506e-4	1	1195.741	2	2479.61	5
355		7	max	.007	3	.021	2	.031	5	4.439e-4	5	NC	5	NC	2
356			min	-.008	2	-.019	3	-.009	1	-1.837e-4	1	1067.597	2	1997.581	5
357		8	max	.007	3	.024	2	.035	5	4.48e-4	5	NC	5	NC	1
358			min	-.008	2	-.021	3	-.007	1	-1.167e-4	1	987.634	2	1663.343	5
359		9	max	.007	3	.026	2	.04	5	4.522e-4	5	NC	5	NC	1
360			min	-.008	2	-.023	3	-.005	1	-4.98e-5	1	940.755	2	1417.056	4
361		10	max	.007	3	.027	2	.045	5	4.624e-4	4	NC	5	NC	1
362			min	-.008	2	-.023	3	-.003	1	1.075e-5	10	919.79	2	1217.712	4
363		11	max	.007	3	.027	2	.05	4	4.812e-4	4	NC	5	NC	1
364			min	-.008	2	-.022	3	-.001	1	1.534e-5	10	922.214	2	1066.913	4
365		12	max	.007	3	.025	2	.056	4	5.001e-4	4	NC	5	NC	1
366			min	-.008	2	-.02	3	0	10	1.994e-5	10	949.183	2	950.302	4
367		13	max	.007	3	.022	2	.061	4	5.189e-4	4	NC	5	NC	2
368			min	-.008	2	-.017	3	0	10	2.454e-5	10	1006.143	2	858.617	4
369		14	max	.007	3	.017	2	.066	4	5.377e-4	4	NC	5	NC	2
370			min	-.008	2	-.014	3	0	10	2.914e-5	10	1105.546	2	785.662	4
371		15	max	.007	3	.011	2	.071	4	5.566e-4	4	NC	4	NC	2
372			min	-.008	2	-.009	3	0	10	3.356e-5	12	1274.626	2	727.172	4
373		16	max	.007	3	.004	2	.075	4	8.342e-4	4	NC	4	NC	2
374			min	-.008	2	-.003	3	0	10	3.442e-5	12	1579.7	2	680.143	4
375		17	max	.007	3	.003	3	.079	4	7.27e-3	4	NC	4	NC	2
376			min	-.008	2	-.005	2	0	10	-7.166e-6	1	2230.614	2	642.462	4
377		18	max	.007	3	.01	3	.083	4	8.719e-3	2	NC	4	NC	1
378			min	-.008	2	-.015	2	0	10	-3.846e-3	3	4317.765	2	612.505	4
379		19	max	.007	3	.018	3	.086	4	1.758e-2	2	NC	1	NC	1
380			min	-.008	2	-.027	2	-.002	1	-7.801e-3	3	NC	1	589.816	4
381	M5	1	max	.022	3	.074	3	.007	5	8.788e-6	4	NC	1	NC	1
382			min	-.027	2	-.071	1	-.004	1	5.224e-8	2	NC	1	NC	1
383		2	max	.022	3	.043	3	.01	5	2.127e-4	5	NC	5	NC	1
384			min	-.027	2	-.04	1	-.004	1	-7.092e-5	1	1473.599	1	NC	1



Company : Schletter, Inc.  
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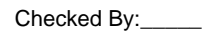
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.022	3	.013	3	.014	5	4.132e-4	5	NC	5	NC	1
386		min	-.027	2	-.01	1	-.004	1	-1.406e-4	1	758.73	2	NC	1
387	4	max	.022	3	.015	2	.018	5	4.299e-4	5	NC	5	NC	1
388		min	-.027	2	-.011	3	-.004	1	-1.343e-4	1	531.185	2	NC	1
389	5	max	.022	3	.037	2	.022	5	4.466e-4	5	NC	5	NC	1
390		min	-.027	2	-.032	3	-.004	1	-1.279e-4	1	421.145	2	NC	1
391	6	max	.022	3	.056	2	.027	5	4.633e-4	5	NC	5	NC	1
392		min	-.027	2	-.048	3	-.004	1	-1.216e-4	1	358.349	2	NC	1
393	7	max	.022	3	.07	2	.032	5	4.8e-4	5	NC	15	NC	1
394		min	-.027	2	-.06	3	-.004	1	-1.153e-4	1	319.697	2	NC	1
395	8	max	.022	3	.081	2	.037	5	4.967e-4	5	NC	15	NC	1
396		min	-.027	2	-.068	3	-.003	1	-1.09e-4	1	295.543	2	NC	1
397	9	max	.022	3	.088	2	.042	5	5.134e-4	5	NC	15	NC	1
398		min	-.027	2	-.073	3	-.003	1	-1.027e-4	1	281.336	2	NC	1
399	10	max	.022	3	.091	2	.048	5	5.301e-4	5	NC	15	NC	1
400		min	-.027	2	-.074	3	-.003	1	-9.634e-5	1	274.913	2	NC	1
401	11	max	.022	3	.089	2	.053	5	5.469e-4	5	NC	15	NC	1
402		min	-.027	2	-.071	3	-.003	1	-9.002e-5	1	275.506	2	NC	1
403	12	max	.022	3	.084	2	.058	5	5.636e-4	5	NC	15	NC	1
404		min	-.027	2	-.065	3	-.003	1	-8.37e-5	1	283.454	2	NC	1
405	13	max	.022	3	.073	2	.063	4	5.803e-4	5	NC	15	NC	1
406		min	-.027	2	-.056	3	-.003	1	-7.738e-5	1	300.38	2	NC	1
407	14	max	.022	3	.058	2	.068	4	5.97e-4	5	NC	5	NC	1
408		min	-.027	2	-.044	3	-.002	1	-7.106e-5	1	330.01	2	NC	1
409	15	max	.021	3	.038	2	.072	4	6.137e-4	5	NC	5	NC	1
410		min	-.027	2	-.029	3	-.002	1	-6.474e-5	1	380.503	2	NC	1
411	16	max	.021	3	.013	2	.076	4	8.86e-4	5	NC	5	NC	1
412		min	-.027	2	-.011	3	-.002	1	-6.25e-5	1	471.776	2	NC	1
413	17	max	.021	3	.01	3	.08	4	7.273e-3	4	NC	5	NC	1
414		min	-.027	2	-.017	2	-.002	1	-1.574e-4	1	667.275	2	NC	1
415	18	max	.021	3	.033	3	.083	4	3.732e-3	4	NC	5	NC	1
416		min	-.027	2	-.052	2	-.002	1	-8.032e-5	1	1292.651	2	NC	1
417	19	max	.021	3	.057	3	.086	4	2.877e-6	5	NC	1	NC	1
418		min	-.027	2	-.089	2	-.002	1	-3.337e-7	3	NC	1	NC	1
419	M9	1	max	.007	.023	.006	.006	5	1.702e-2	3	NC	1	NC	1
420		min	-.007	2	-.021	2	-.005	1	-1.424e-2	1	NC	1	NC	1
421	2	max	.007	3	.013	.005	.005	5	8.42e-3	3	NC	4	NC	1
422		min	-.007	2	-.012	2	-.001	1	-6.992e-3	1	4918.15	2	NC	1
423	3	max	.007	3	.004	.006	.006	4	1.19e-4	1	NC	4	NC	2
424		min	-.007	2	-.003	2	0	3	-2.036e-5	5	2526.527	2	8110.804	1
425	4	max	.007	3	.005	.007	.007	4	6.397e-5	1	NC	4	NC	2
426		min	-.008	2	-.004	3	-.001	3	-2.635e-5	3	1770.428	2	6737.245	1
427	5	max	.007	3	.011	.009	.009	4	2.318e-5	2	NC	4	NC	2
428		min	-.008	2	-.01	3	-.002	3	-3.37e-5	3	1404.914	2	6500.565	1
429	6	max	.007	3	.017	.012	.012	4	7.783e-6	10	NC	5	NC	2
430		min	-.008	2	-.015	3	-.002	3	-4.607e-5	1	1196.426	2	6253.833	4
431	7	max	.007	3	.021	.016	.016	4	3.178e-6	10	NC	5	NC	2
432		min	-.008	2	-.019	3	-.003	3	-1.011e-4	1	1068.2	2	4278.552	4
433	8	max	.007	3	.024	.02	.02	4	-1.427e-6	10	NC	5	NC	1
434		min	-.008	2	-.021	3	-.003	3	-1.561e-4	1	988.183	2	3130.692	4
435	9	max	.007	3	.026	.024	.024	5	-6.033e-6	10	NC	5	NC	1
436		min	-.008	2	-.023	3	-.003	3	-2.111e-4	1	941.267	2	2404.818	4
437	10	max	.007	3	.027	.03	.03	5	-7.319e-6	15	NC	5	NC	1
438		min	-.008	2	-.023	3	-.004	3	-2.662e-4	1	920.281	2	1916.334	4
439	11	max	.007	3	.027	.035	.035	5	-6.378e-6	15	NC	5	NC	1
440		min	-.008	2	-.022	3	-.005	1	-3.212e-4	1	922.696	2	1571.602	4
441	12	max	.007	3	.025	.041	.041	5	-5.437e-6	15	NC	5	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.128	3	.048	1	6.433e-3	2	NC	5	NC	3
500		min	-.086	4	-.274	2	0	10	-4.215e-3	3	581.34	2	2747.918	1
501	4	max	.002	1	.161	3	.072	1	7.535e-3	2	NC	5	NC	3
502		min	-.086	4	-.346	2	0	10	-4.91e-3	3	451.335	2	1875.441	1
503	5	max	.002	1	.172	3	.083	1	8.637e-3	2	NC	5	NC	10
504		min	-.086	4	-.369	2	0	10	-5.605e-3	3	420.783	2	1642.25	1
505	6	max	.002	1	.163	3	.077	1	9.739e-3	2	NC	5	NC	5
506		min	-.086	4	-.345	2	-.002	10	-6.299e-3	3	452.828	2	1761.883	1
507	7	max	.002	1	.138	3	.056	1	1.084e-2	2	NC	5	NC	2
508		min	-.086	4	-.282	2	-.006	10	-6.994e-3	3	563.044	2	2392.354	1
509	8	max	.002	1	.104	3	.025	1	1.194e-2	2	NC	5	NC	2
510		min	-.086	4	-.2	2	-.01	10	-7.689e-3	3	830.575	2	4967.071	1
511	9	max	.002	1	.072	3	.023	3	1.304e-2	2	NC	4	NC	1
512		min	-.086	4	-.124	2	-.021	2	-8.384e-3	3	1479.688	2	8880.776	3
513	10	max	.002	1	.057	3	.021	3	1.415e-2	2	NC	4	NC	4
514		min	-.086	4	-.089	2	-.027	2	-9.079e-3	3	2300.783	2	7488.857	2
515	11	max	.002	1	.072	3	.021	3	1.304e-2	2	NC	4	NC	1
516		min	-.086	4	-.124	2	-.021	2	-8.383e-3	3	1479.688	2	NC	1
517	12	max	.002	1	.104	3	.025	1	1.194e-2	2	NC	5	NC	2
518		min	-.086	4	-.2	2	-.01	10	-7.687e-3	3	830.575	2	4929.958	1
519	13	max	.002	1	.138	3	.056	1	1.084e-2	2	NC	5	NC	2
520		min	-.086	4	-.282	2	-.006	10	-6.992e-3	3	563.044	2	2389.819	1
521	14	max	.002	1	.163	3	.077	1	9.74e-3	2	NC	5	NC	3
522		min	-.086	4	-.345	2	-.002	10	-6.296e-3	3	452.828	2	1765.553	1
523	15	max	.002	1	.172	3	.083	1	8.638e-3	2	NC	5	NC	3
524		min	-.086	4	-.369	2	-.002	5	-5.6e-3	3	420.783	2	1649.979	1
525	16	max	.002	1	.161	3	.072	1	7.536e-3	2	NC	5	NC	3
526		min	-.086	4	-.346	2	-.006	5	-4.905e-3	3	451.335	2	1889.875	1
527	17	max	.002	1	.128	3	.047	1	6.434e-3	2	NC	5	NC	3
528		min	-.086	4	-.274	2	-.008	5	-4.209e-3	3	581.34	2	2781.069	1
529	18	max	.002	1	.078	3	.018	1	5.333e-3	2	NC	5	NC	2
530		min	-.086	4	-.163	2	-.007	5	-3.513e-3	3	1059.454	2	6507.614	1
531	19	max	.002	1	.018	3	.007	3	4.231e-3	2	NC	1	NC	1
532		min	-.086	4	-.027	2	-.008	2	-2.818e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.516e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.873e-4	5	NC	1	NC	1
535	2	max	0	3	0	15	.007	4	8.372e-4	3	NC	1	NC	1
536		min	0	5	-.007	1	0	3	-6.115e-4	5	NC	1	NC	1
537	3	max	0	3	0	15	.016	4	1.323e-3	3	NC	5	NC	1
538		min	-.001	5	-.014	1	-.003	3	-1.084e-3	2	5896.947	1	5005.844	4
539	4	max	0	3	0	15	.025	4	1.808e-3	3	NC	5	NC	9
540		min	-.002	5	-.02	1	-.007	3	-1.596e-3	2	4045.65	1	3242.601	4
541	5	max	0	3	0	15	.033	4	2.294e-3	3	NC	5	NC	9
542		min	-.003	5	-.026	1	-.011	3	-2.107e-3	1	3156.862	1	2441.708	4
543	6	max	0	3	0	15	.04	4	2.78e-3	3	NC	5	8473.263	9
544		min	-.003	5	-.031	1	-.016	3	-2.624e-3	1	2656.833	1	2018.125	4
545	7	max	0	3	0	15	.045	4	3.265e-3	3	NC	5	6675.375	9
546		min	-.004	5	-.035	1	-.021	3	-3.141e-3	1	2356.131	1	1784.578	4
547	8	max	0	3	0	15	.048	4	3.751e-3	3	NC	5	5536.837	9
548		min	-.005	5	-.038	1	-.026	3	-3.658e-3	1	2175.665	1	1666.731	4
549	9	max	0	3	0	15	.049	4	4.237e-3	3	NC	5	4788.327	9
550		min	-.006	5	-.04	1	-.031	3	-4.175e-3	1	2078.528	1	1633.827	4
551	10	max	0	3	0	15	.048	4	4.722e-3	3	NC	5	4293.455	9
552		min	-.006	5	-.04	1	-.035	3	-4.692e-3	1	2047.798	1	1677.571	4
553	11	max	0	3	.001	15	.044	4	5.208e-3	3	NC	5	3980.366	9
554		min	-.007	5	-.04	1	-.037	3	-5.209e-3	1	2078.528	1	1568.185	3
555	12	max	0	3	.001	15	.039	4	5.693e-3	3	NC	5	3813.693	9



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.008	5	-0.038	1	-0.038	3	-5.726e-3	1	2175.665	1	1498.558	3
557	13	max	0	3	.002	5	.037	1	6.179e-3	3	NC	5	4741.083	15
558		min	-0.008	5	-0.035	1	-0.037	3	-6.243e-3	1	2356.131	1	1483.59	3
559	14	max	0	3	.002	5	.034	1	6.665e-3	3	NC	5	6756.111	15
560		min	-0.009	5	-0.032	1	-0.034	3	-6.759e-3	1	2656.833	1	1529.74	3
561	15	max	0	3	.003	5	.029	1	7.15e-3	3	NC	5	NC	15
562		min	-0.01	5	-0.027	1	-0.028	3	-7.276e-3	1	3156.862	1	1660.766	3
563	16	max	.001	3	.004	5	.021	1	7.636e-3	3	NC	5	NC	5
564		min	-0.01	5	-0.022	1	-0.02	3	-7.793e-3	1	4045.65	1	1941.197	3
565	17	max	.001	3	.004	5	.01	1	8.122e-3	3	NC	5	NC	4
566		min	-0.011	5	-0.015	1	-0.008	3	-8.31e-3	1	5896.947	1	2573.492	3
567	18	max	.001	3	.005	5	.007	3	8.607e-3	3	NC	1	NC	4
568		min	-0.012	5	-0.009	1	-0.011	2	-8.827e-3	1	NC	1	4581.853	3
569	19	max	.001	3	.006	5	.026	3	9.093e-3	3	NC	1	NC	1
570		min	-0.012	5	-0.002	9	-0.029	2	-9.344e-3	1	NC	1	NC	1
571	M16A	1	max	0	10	0	.008	3	2.677e-3	3	NC	1	NC	1
572		min	-0.004	4	-0.003	4	-0.008	2	-2.662e-3	2	NC	1	NC	1
573	2	max	0	10	-0.003	12	.003	9	2.567e-3	3	NC	1	NC	1
574		min	-0.004	4	-0.015	4	-0.003	5	-2.543e-3	2	6845.276	4	NC	1
575	3	max	0	10	-0.007	12	.009	1	2.457e-3	3	NC	12	NC	4
576		min	-0.004	4	-0.026	4	-0.008	5	-2.424e-3	2	3483.327	4	6068.951	1
577	4	max	0	10	-0.01	12	.013	1	2.347e-3	3	8091.3	12	NC	10
578		min	-0.004	4	-0.037	4	-0.015	5	-2.305e-3	2	2389.766	4	4609.509	1
579	5	max	0	10	-0.013	12	.016	1	2.237e-3	3	6313.723	12	NC	10
580		min	-0.003	4	-0.046	4	-0.023	5	-2.186e-3	2	1864.759	4	3638.196	5
581	6	max	0	10	-0.015	12	.018	1	2.127e-3	3	5313.666	12	NC	10
582		min	-0.003	4	-0.054	4	-0.032	5	-2.067e-3	2	1569.391	4	2596.07	5
583	7	max	0	10	-0.017	12	.019	1	2.017e-3	3	4712.263	12	NC	10
584		min	-0.003	4	-0.06	4	-0.04	5	-1.948e-3	2	1391.767	4	2044.389	5
585	8	max	0	10	-0.018	12	.019	1	1.908e-3	3	4351.331	12	NC	10
586		min	-0.003	4	-0.065	4	-0.048	5	-1.829e-3	2	1285.166	4	1729.095	5
587	9	max	0	10	-0.019	12	.018	1	1.798e-3	3	4157.055	12	NC	10
588		min	-0.002	4	-0.068	4	-0.053	5	-1.71e-3	2	1227.787	4	1547.46	5
589	10	max	0	10	-0.02	12	.016	1	1.688e-3	3	4095.596	12	NC	10
590		min	-0.002	4	-0.068	4	-0.056	5	-1.591e-3	2	1209.635	4	1453.407	5
591	11	max	0	10	-0.019	12	.014	1	1.578e-3	3	4157.055	12	NC	10
592		min	-0.002	4	-0.067	4	-0.057	5	-1.472e-3	2	1227.787	4	1426.857	5
593	12	max	0	10	-0.018	12	.012	1	1.468e-3	3	4351.331	12	NC	9
594		min	-0.002	4	-0.064	4	-0.056	5	-1.353e-3	2	1285.166	4	1463.095	5
595	13	max	0	10	-0.017	12	.009	1	1.358e-3	3	4712.263	12	NC	2
596		min	-0.001	4	-0.059	4	-0.052	5	-1.234e-3	2	1391.767	4	1570.795	5
597	14	max	0	10	-0.015	12	.007	1	1.248e-3	3	5313.666	12	NC	2
598		min	-0.001	4	-0.052	4	-0.046	5	-1.116e-3	2	1569.391	4	1777.096	5
599	15	max	0	10	-0.013	12	.004	1	1.138e-3	3	6313.723	12	NC	1
600		min	0	4	-0.044	4	-0.038	5	-9.966e-4	2	1864.759	4	2145.763	5
601	16	max	0	10	-0.01	12	.002	1	1.029e-3	3	8091.3	12	NC	1
602		min	0	4	-0.034	4	-0.029	5	-8.776e-4	2	2389.766	4	2835.697	5
603	17	max	0	10	-0.007	12	0	9	9.186e-4	3	NC	12	NC	1
604		min	0	4	-0.024	4	-0.019	5	-7.587e-4	2	3483.327	4	4339.784	5
605	18	max	0	10	-0.003	12	0	3	9.057e-4	4	NC	1	NC	1
606		min	0	4	-0.012	4	-0.009	5	-6.397e-4	2	6845.276	4	9170.104	5
607	19	max	0	1	0	1	0	1	9.752e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.208e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

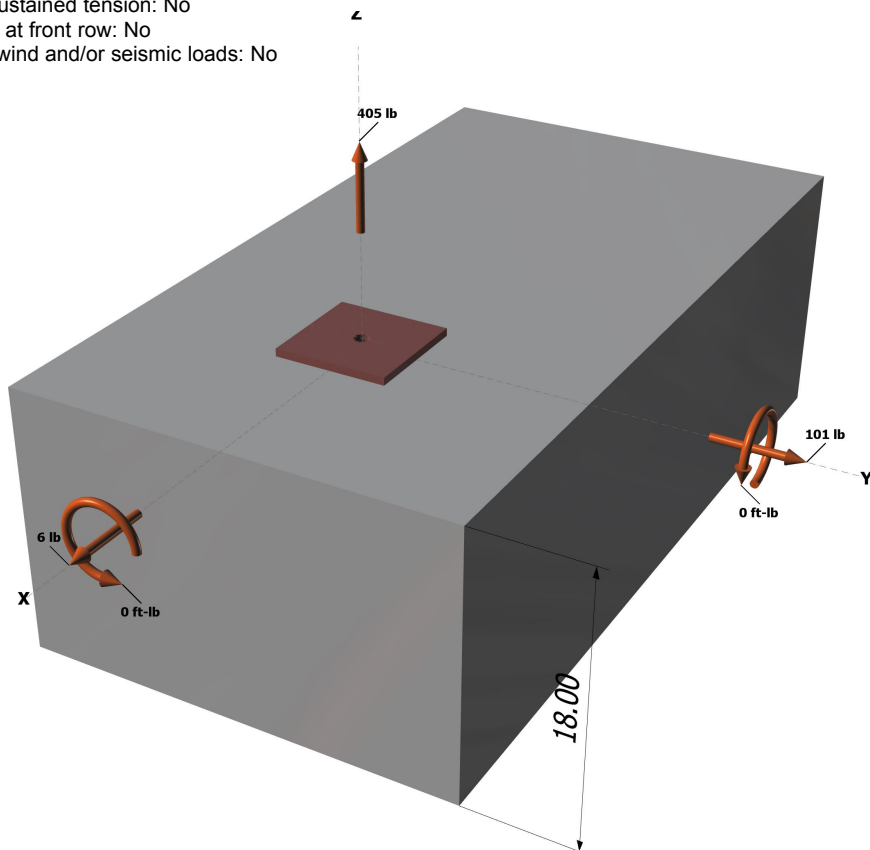
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



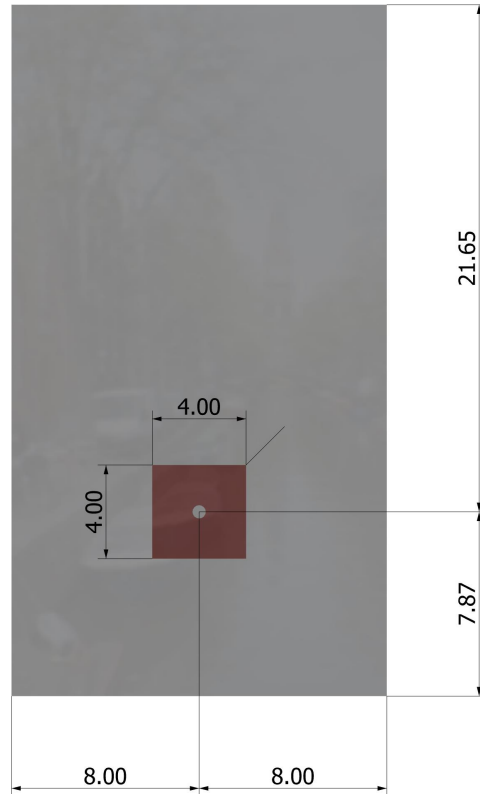
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

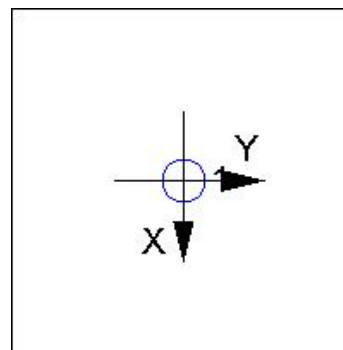
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

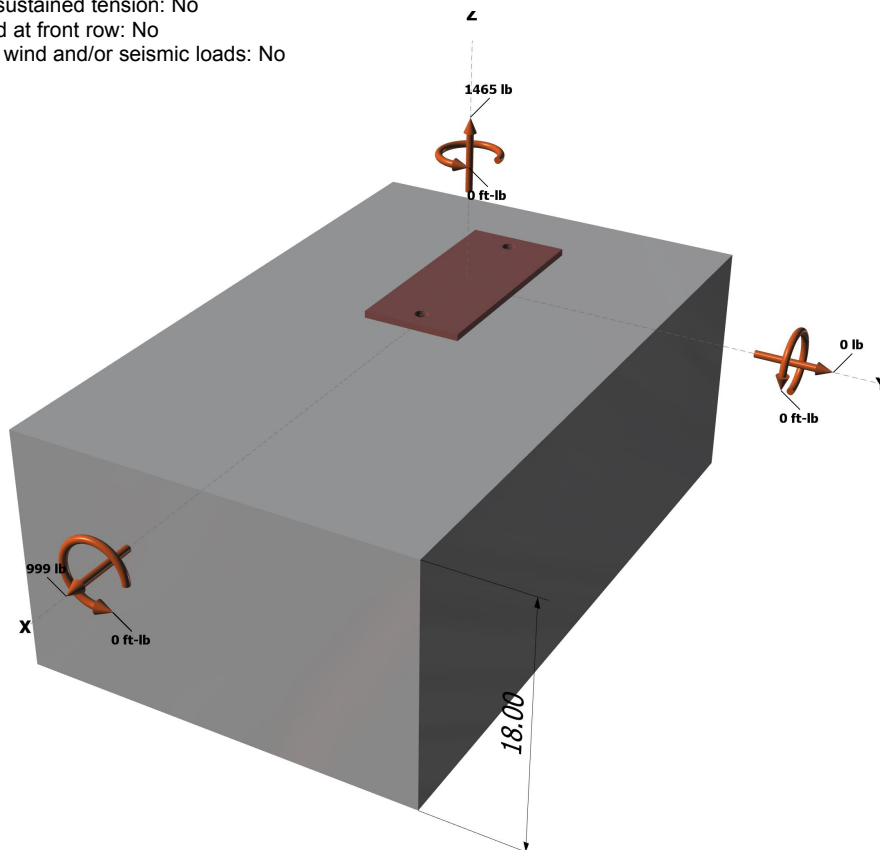
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



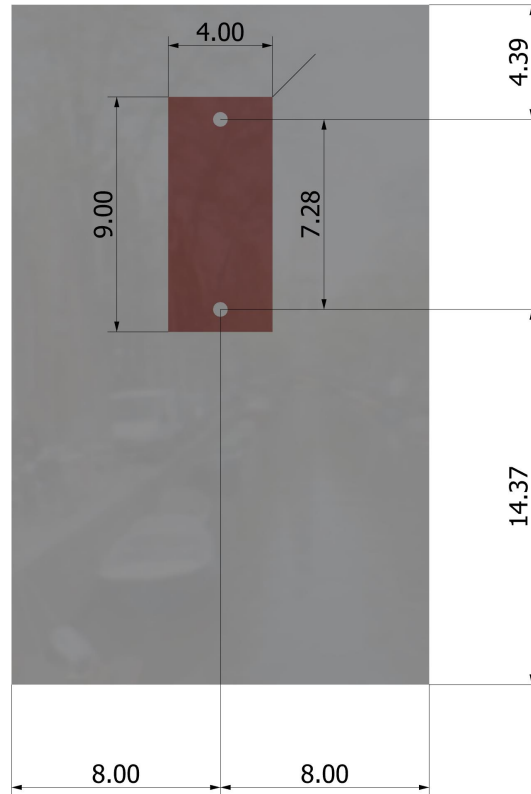
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.