

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	115 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 20.76 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

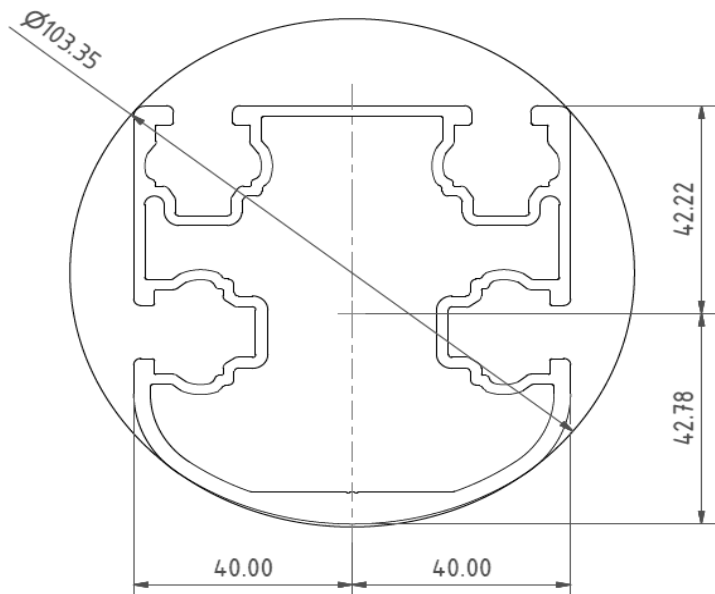
Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	135 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.785 k-ft
$M_z$ =	0.418 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>100%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.862 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.895 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>84%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

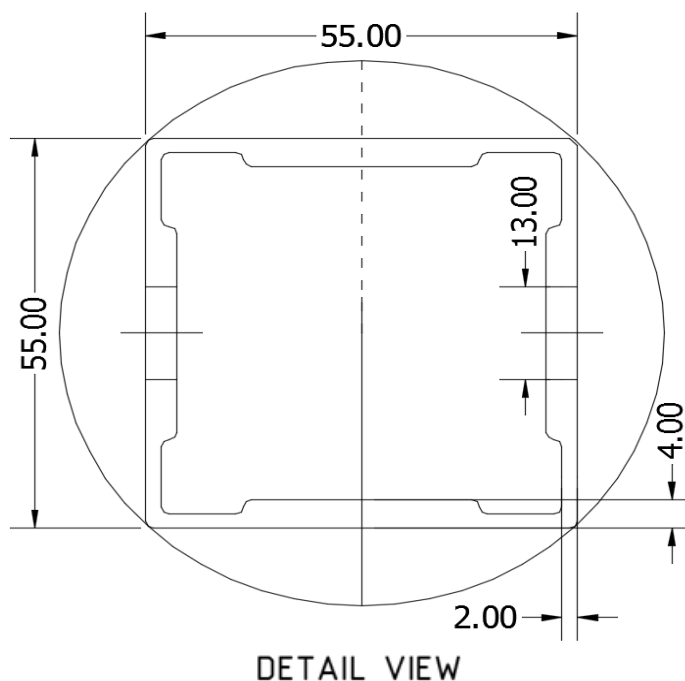
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.638 k-ft
$P_n$ =	0.179 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>46%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.669 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>37%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.049 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>35%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

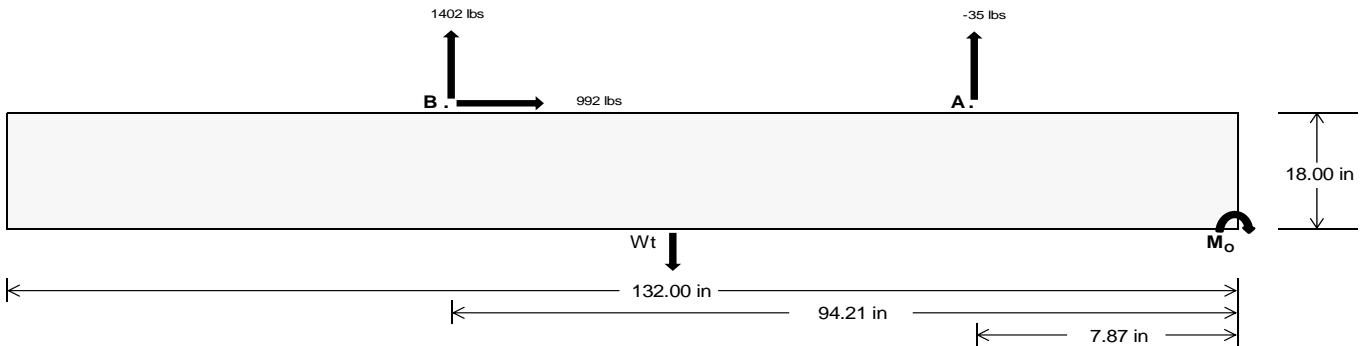
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>48.23</b>	<b>6095.32</b>	k
Compressive Load =	<b>3147.24</b>	<b>4868.49</b>	k
Lateral Load =	<b>433.19</b>	<b>4301.55</b>	k
Moment (Weak Axis) =	<b>0.83</b>	<b>0.27</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 149644.2$  in-lbs  
Resisting Force Required = 2267.34 lbs  
S.F. = 1.67  
Weight Required = 3778.89 lbs  
Minimum Width = 30 in  
Weight Provided = 5981.25 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 30in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 991.91 lbs  
Friction = 0.4  
Weight Required = 2479.79 lbs  
Resisting Weight = 5981.25 lbs  
Additional Weight Required = 0 lbs

Use a 132in long x 30in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 991.91 lbs  
Cohesion = 130 psf  
Area = 27.50 ft<sup>2</sup>  
Resisting = 2990.63 lbs  
Additional Weight Required = 0 lbs

Use a 132in long x 30in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.5 \text{ ft}) =$   
30 in 31 in 32 in 33 in  
5981 lbs 6181 lbs 6380 lbs 6579 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in
$F_A$	1205 lbs	1205 lbs	1205 lbs	1205 lbs	1030 lbs	1030 lbs	1030 lbs	1030 lbs	1529 lbs	1529 lbs	1529 lbs	1529 lbs	71 lbs	71 lbs	71 lbs	71 lbs
$F_B$	1088 lbs	1088 lbs	1088 lbs	1088 lbs	2074 lbs	2074 lbs	2074 lbs	2074 lbs	2239 lbs	2239 lbs	2239 lbs	2239 lbs	-2804 lbs	-2804 lbs	-2804 lbs	-2804 lbs
$F_V$	200 lbs	200 lbs	200 lbs	200 lbs	1824 lbs	1824 lbs	1824 lbs	1824 lbs	1495 lbs	1495 lbs	1495 lbs	1495 lbs	-1984 lbs	-1984 lbs	-1984 lbs	-1984 lbs
$P_{total}$	8274 lbs	8474 lbs	8673 lbs	8872 lbs	9085 lbs	9285 lbs	9484 lbs	9683 lbs	9749 lbs	9948 lbs	10148 lbs	10347 lbs	856 lbs	976 lbs	1095 lbs	1215 lbs
$M$	3576 lbs-ft	3576 lbs-ft	3576 lbs-ft	3576 lbs-ft	2850 lbs-ft	2850 lbs-ft	2850 lbs-ft	2850 lbs-ft	4383 lbs-ft	4383 lbs-ft	4383 lbs-ft	4383 lbs-ft	3959 lbs-ft	3959 lbs-ft	3959 lbs-ft	3959 lbs-ft
$e$	0.43 ft	0.42 ft	0.41 ft	0.40 ft	0.31 ft	0.31 ft	0.30 ft	0.29 ft	0.45 ft	0.44 ft	0.43 ft	0.42 ft	4.63 ft	4.06 ft	3.61 ft	3.26 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	230.0 psf	229.6 psf	229.2 psf	228.8 psf	273.8 psf	272.0 psf	270.3 psf	268.7 psf	267.6 psf	266.0 psf	264.4 psf	263.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	371.8 psf	366.8 psf	362.2 psf	357.8 psf	386.9 psf	381.4 psf	376.3 psf	371.5 psf	441.4 psf	434.2 psf	427.4 psf	421.1 psf	260.9 psf	174.6 psf	145.2 psf	131.4 psf

Maximum Bearing Pressure = 441 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 30in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

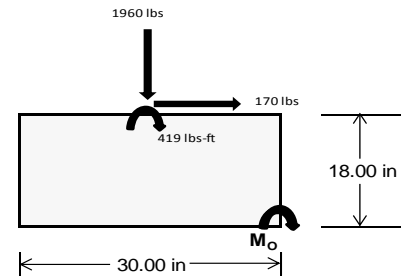
### Overturning Check

$M_o = 1775.5 \text{ ft-lbs}$   
 Resisting Force Required = 1420.42 lbs  
 S.F. = 1.67  
 Weight Required = 2367.37 lbs  
 Minimum Width = 30 in  
 Weight Provided = 5981.25 lbs

*A minimum 132in long x 30in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	30 in			30 in			30 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	335 lbs	693 lbs	224 lbs	772 lbs	1960 lbs	687 lbs	137 lbs	203 lbs	27 lbs
$F_v$	236 lbs	231 lbs	242 lbs	172 lbs	170 lbs	189 lbs	237 lbs	232 lbs	239 lbs
$P_{total}$	7740 lbs	8098 lbs	7629 lbs	7821 lbs	9009 lbs	7736 lbs	2302 lbs	2368 lbs	2192 lbs
$M$	904 lbs-ft	892 lbs-ft	924 lbs-ft	669 lbs-ft	675 lbs-ft	723 lbs-ft	904 lbs-ft	890 lbs-ft	911 lbs-ft
$e$	0.12 ft	0.11 ft	0.12 ft	0.09 ft	0.07 ft	0.09 ft	0.39 ft	0.38 ft	0.42 ft
$L/6$	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft
$f_{min}$	202.5 psf	216.6 psf	196.8 psf	226.0 psf	268.7 psf	218.2 psf	4.8 psf	8.4 psf	0.2 psf
$f_{max}$	360.4 psf	372.4 psf	358.0 psf	342.8 psf	386.5 psf	344.4 psf	162.6 psf	163.8 psf	159.2 psf



Maximum Bearing Pressure = 386 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 30in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 132in long x 30in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.523 k
Allowable Uplift =	1.214 k
Utilization =	<u>43%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.097 k
Allowable Uplift =	4.357 k
Utilization =	<u>48%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.421 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>33%</u>

#### Rear Strut

Maximum Axial Load =	4.014 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>54%</u>

#### Diagonal Strut

Maximum Axial Load =	2.710 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>37%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u>0.827 ≤ 1.076, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 135 \text{ in}$$

$$J = 0.432$$

$$373.473$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 135$$

$$J = 0.432$$

$$237.507$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-69.488	-69.488	0	0
2	M14	y	-69.488	-69.488	0	0
3	M15	y	-115.813	-115.813	0	0
4	M16	y	-115.813	-115.813	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	156.347	156.347	0	0
2	M14	y	121.603	121.603	0	0
3	M15	y	69.488	69.488	0	0
4	M16	y	69.488	69.488	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 35° 115mph 30psf 11.25ft 7-10.r3d]Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	140.267	1	638.963	12	215.816	1	.012	2	.419	1	1.507	2
20			min	9.029	12	-1068.508	3	-134.43	14	-.002	3	.02	12	-2.405	3
21		11	max	140.267	1	539.476	2	-8.799	12	.012	2	.178	1	.76	2
22			min	9.029	12	-878.561	3	-169.503	1	-.002	3	.007	12	-1.188	3
23		12	max	140.267	1	423.609	2	-6.238	12	.012	2	.074	5	.158	2
24			min	9.029	12	-688.615	3	-123.189	1	-.002	3	-.004	1	-.208	3
25		13	max	140.267	1	307.741	2	-3.676	12	.012	2	.031	5	.534	3
26			min	9.029	12	-498.668	3	-76.876	1	-.002	3	-.13	1	-.299	2
27		14	max	140.267	1	191.874	2	-1.115	12	.012	2	-.003	15	1.038	3
28			min	9.029	12	-308.721	3	-35.853	4	-.002	3	-.197	1	-.611	2
29		15	max	140.267	1	76.007	2	15.751	1	.012	2	-.011	12	1.306	3
30			min	2.91	15	-118.774	3	-23.847	5	-.002	3	-.206	1	-.779	2
31		16	max	140.267	1	71.172	3	62.065	1	.012	2	-.008	12	1.335	3
32			min	-8.104	5	-39.861	2	-19.884	5	-.002	3	-.157	1	-.801	2
33		17	max	140.267	1	261.119	3	108.379	1	.012	2	-.001	12	1.128	3
34			min	-20.936	5	-155.728	2	-15.921	5	-.002	3	-.102	4	-.679	2
35		18	max	140.267	1	451.066	3	154.692	1	.012	2	.114	1	.683	3
36			min	-33.767	5	-271.595	2	-11.958	5	-.002	3	-.105	5	-.412	2
37		19	max	140.267	1	641.012	3	201.006	1	.012	2	.336	1	0	2
38			min	-46.599	5	-387.463	2	-7.995	5	-.002	3	-.118	5	0	3
39	M14	1	max	67.808	4	406.481	2	-11.99	12	.007	3	.378	1	0	4
40			min	3.686	12	-505.979	3	-206.66	1	-.009	2	.024	12	0	3
41		2	max	60.567	1	290.614	2	-9.428	12	.007	3	.217	4	.541	3
42			min	3.686	12	-359.462	3	-160.347	1	-.009	2	.011	12	-.436	2
43		3	max	60.567	1	174.746	2	-6.867	12	.007	3	.115	5	.899	3
44			min	3.686	12	-212.945	3	-114.033	1	-.009	2	-.023	1	-.727	2
45		4	max	60.567	1	58.879	2	-4.305	12	.007	3	.059	5	1.073	3
46			min	3.686	12	-66.429	3	-67.72	1	-.009	2	-.136	1	-.873	2
47		5	max	60.567	1	80.088	3	-1.744	12	.007	3	.008	5	1.065	3
48			min	.756	15	-56.988	2	-45.05	4	-.009	2	-.192	1	-.874	2
49		6	max	60.567	1	226.605	3	24.907	1	.007	3	-.011	12	.873	3
50			min	-11.619	5	-172.856	2	-34.943	5	-.009	2	-.19	1	-.73	2
51		7	max	60.567	1	373.122	3	71.221	1	.007	3	-.008	12	.498	3
52			min	-24.45	5	-288.723	2	-30.98	5	-.009	2	-.13	1	-.442	2
53		8	max	60.567	1	519.638	3	117.534	1	.007	3	0	10	.006	9
54			min	-37.282	5	-404.59	2	-27.017	5	-.009	2	-.121	4	-.06	3
55		9	max	60.567	1	666.155	3	163.848	1	.007	3	.164	1	.57	2
56			min	-50.113	5	-520.458	2	-23.054	5	-.009	2	-.147	5	-.801	3
57		10	max	88.154	4	812.672	3	210.161	1	.009	2	.398	1	1.293	2
58			min	3.686	12	-636.325	2	-137.42	14	-.007	3	.019	12	-1.725	3
59		11	max	75.322	4	520.458	2	-8.501	12	.009	2	.218	4	.57	2
60			min	3.686	12	-666.155	3	-163.848	1	-.007	3	.007	12	-.801	3
61		12	max	62.491	4	404.59	2	-5.94	12	.009	2	.113	5	.006	9
62			min	3.686	12	-519.638	3	-117.534	1	-.007	3	-.012	1	-.06	3
63		13	max	60.567	1	288.723	2	-3.379	12	.009	2	.057	5	.498	3
64			min	3.686	12	-373.122	3	-71.221	1	-.007	3	-.13	1	-.442	2
65		14	max	60.567	1	172.856	2	-.817	12	.009	2	.005	5	.873	3
66			min	3.686	12	-226.605	3	-45.99	4	-.007	3	-.19	1	-.73	2
67		15	max	60.567	1	56.988	2	21.406	1	.009	2	-.01	12	1.065	3
68			min	3.686	12	-80.088	3	-35.181	5	-.007	3	-.192	1	-.874	2
69		16	max	60.567	1	66.429	3	67.72	1	.009	2	-.006	12	1.073	3
70			min	-4.246	5	-58.879	2	-31.219	5	-.007	3	-.136	1	-.873	2
71		17	max	60.567	1	212.945	3	114.033	1	.009	2	.001	3	.899	3
72			min	-17.078	5	-174.746	2	-27.256	5	-.007	3	-.127	4	-.727	2
73		18	max	60.567	1	359.462	3	160.347	1	.009	2	.149	1	.541	3
74			min	-29.909	5	-290.614	2	-23.293	5	-.007	3	-.151	5	-.436	2
75		19	max	60.567	1	505.979	3	206.66	1	.009	2	.378	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-42.741	5	-406.481	2	-19.33	5	-.007	3	-.177	5	0	3
77	M15	1	max	94.239	5	601.927	2	-11.944	12	.009	2	.393	4	0	2
78			min	-63.898	1	-286.207	3	-206.628	1	-.007	3	.024	12	0	3
79		2	max	81.407	5	428.154	2	-9.382	12	.009	2	.257	4	.307	3
80			min	-63.898	1	-204.834	3	-160.314	1	-.007	3	.01	12	-.644	2
81		3	max	68.576	5	254.38	2	-6.821	12	.009	2	.144	5	.512	3
82			min	-63.898	1	-123.461	3	-114.001	1	-.007	3	-.023	1	-1.07	2
83		4	max	55.744	5	80.606	2	-4.26	12	.009	2	.077	5	.616	3
84			min	-63.898	1	-42.088	3	-69.574	4	-.007	3	-.136	1	-1.28	2
85		5	max	42.913	5	39.285	3	-1.698	12	.009	2	.014	5	.617	3
86			min	-63.898	1	-93.167	2	-53.948	4	-.007	3	-.192	1	-1.272	2
87		6	max	30.081	5	120.658	3	24.94	1	.009	2	-.011	12	.517	3
88			min	-63.898	1	-266.941	2	-43.791	5	-.007	3	-.19	1	-1.047	2
89		7	max	17.25	5	202.031	3	71.253	1	.009	2	-.008	12	.316	3
90			min	-63.898	1	-440.715	2	-39.828	5	-.007	3	-.13	1	-.605	2
91		8	max	4.418	5	283.404	3	117.567	1	.009	2	0	10	.055	2
92			min	-63.898	1	-614.488	2	-35.865	5	-.007	3	-.148	4	0	15
93		9	max	-4.198	12	364.777	3	163.88	1	.009	2	.164	1	.932	2
94			min	-63.898	1	-788.262	2	-31.902	5	-.007	3	-.185	5	-.393	3
95		10	max	-4.198	12	962.035	2	32.472	10	.007	3	.398	1	2.026	2
96			min	-63.898	1	-536.836	10	-210.194	1	-.009	2	.019	12	-.9	3
97		11	max	-1.885	15	788.262	2	-8.547	12	.007	3	.257	4	.932	2
98			min	-63.898	1	-364.777	3	-163.88	1	-.009	2	.007	12	-.393	3
99		12	max	-4.198	12	614.488	2	-5.986	12	.007	3	.14	5	.055	2
100			min	-63.898	1	-283.404	3	-117.567	1	-.009	2	-.012	1	0	15
101		13	max	-4.198	12	440.715	2	-3.425	12	.007	3	.073	5	.316	3
102			min	-63.898	1	-202.031	3	-71.253	1	-.009	2	-.13	1	-.605	2
103		14	max	-4.198	12	266.941	2	-.863	12	.007	3	.01	5	.517	3
104			min	-63.898	1	-120.658	3	-54.921	4	-.009	2	-.19	1	-1.047	2
105		15	max	-4.198	12	93.167	2	21.374	1	.007	3	-.01	12	.617	3
106			min	-71.42	4	-39.285	3	-44.036	5	-.009	2	-.192	1	-1.272	2
107		16	max	-4.198	12	42.088	3	67.687	1	.007	3	-.007	12	.616	3
108			min	-84.252	4	-80.606	2	-40.073	5	-.009	2	-.136	1	-1.28	2
109		17	max	-4.198	12	123.461	3	114.001	1	.007	3	0	3	.512	3
110			min	-97.083	4	-254.38	2	-36.11	5	-.009	2	-.155	4	-1.07	2
111		18	max	-4.198	12	204.834	3	160.314	1	.007	3	.149	1	.307	3
112			min	-109.915	4	-428.154	2	-32.147	5	-.009	2	-.19	5	-.644	2
113		19	max	-4.198	12	286.207	3	206.628	1	.007	3	.378	1	0	2
114			min	-122.746	4	-601.927	2	-28.184	5	-.009	2	-.228	5	0	5
115	M16	1	max	91.815	5	583.582	2	-11.544	12	.009	2	.338	1	0	2
116			min	-151.279	1	-271.481	3	-201.247	1	-.01	3	.021	12	0	3
117		2	max	78.983	5	409.808	2	-8.983	12	.009	2	.201	4	.288	3
118			min	-151.279	1	-190.108	3	-154.933	1	-.01	3	.008	12	-.621	2
119		3	max	66.152	5	236.035	2	-6.421	12	.009	2	.111	5	.475	3
120			min	-151.279	1	-108.735	3	-108.62	1	-.01	3	-.05	1	-1.025	2
121		4	max	53.32	5	62.261	2	-3.86	12	.009	2	.058	5	.56	3
122			min	-151.279	1	-27.362	3	-62.306	1	-.01	3	-.157	1	-1.211	2
123		5	max	40.489	5	54.01	3	-1.298	12	.009	2	.011	5	.544	3
124			min	-151.279	1	-111.512	2	-40.959	4	-.01	3	-.206	1	-1.18	2
125		6	max	27.657	5	135.383	3	30.321	1	.009	2	-.011	12	.425	3
126			min	-151.279	1	-285.286	2	-32.342	5	-.01	3	-.197	1	-.932	2
127		7	max	14.826	5	216.756	3	76.635	1	.009	2	-.008	12	.205	3
128			min	-151.279	1	-459.06	2	-28.379	5	-.01	3	-.13	1	-.467	2
129		8	max	1.994	5	298.129	3	122.948	1	.009	2	0	10	.215	2
130			min	-151.279	1	-632.833	2	-24.416	5	-.01	3	-.106	4	-.117	3
131		9	max	-7.018	15	379.502	3	169.262	1	.009	2	.178	1	1.115	2
132			min	-151.279	1	-806.607	2	-20.453	5	-.01	3	-.131	5	-.54	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-9.421	12	782.049	1	142.779	9	.01	3	.418	1	2.232	2
134		min	-151.279	1	-980.381	2	-215.575	1	-.009	2	.021	12	-1.065	3
135	11	max	-9.421	12	806.607	2	-8.947	12	.01	3	.208	4	1.115	2
136		min	-151.279	1	-379.502	3	-169.262	1	-.009	2	.008	12	-.54	3
137	12	max	-9.421	12	632.833	2	-6.386	12	.01	3	.102	4	.215	2
138		min	-151.279	1	-298.129	3	-122.948	1	-.009	2	-.005	1	-.117	3
139	13	max	-9.421	12	459.06	2	-3.824	12	.01	3	.048	5	.205	3
140		min	-151.279	1	-216.756	3	-76.635	1	-.009	2	-.13	1	-.467	2
141	14	max	-9.421	12	285.286	2	-1.263	12	.01	3	0	15	.425	3
142		min	-151.279	1	-135.383	3	-45.606	4	-.009	2	-.197	1	-.932	2
143	15	max	-9.421	12	111.512	2	15.992	1	.01	3	-.011	12	.544	3
144		min	-151.279	1	-54.01	3	-33.525	5	-.009	2	-.206	1	-1.18	2
145	16	max	-9.421	12	27.362	3	62.306	1	.01	3	-.008	12	.56	3
146		min	-151.279	1	-62.261	2	-29.562	5	-.009	2	-.157	1	-1.211	2
147	17	max	-9.421	12	108.735	3	108.62	1	.01	3	-.002	12	.475	3
148		min	-151.279	1	-236.035	2	-25.599	5	-.009	2	-.134	4	-1.025	2
149	18	max	-9.421	12	190.108	3	154.933	1	.01	3	.115	1	.288	3
150		min	-151.279	1	-409.808	2	-21.636	5	-.009	2	-.149	5	-.621	2
151	19	max	-9.421	12	271.481	3	201.247	1	.01	3	.338	1	0	2
152		min	-157.173	4	-583.582	2	-17.674	5	-.009	2	-.174	5	0	3
153	M2	1	max	939.165	2	2.039	4	.475	1	0	0	3	0	1
154		min	-1221.32	3	.489	15	-29.851	4	0	4	0	2	0	1
155	2	max	939.686	2	1.92	4	.475	1	0	12	0	1	0	15
156		min	-1220.929	3	.461	15	-30.309	4	0	4	-.011	4	0	4
157	3	max	940.207	2	1.801	4	.475	1	0	12	0	1	0	15
158		min	-1220.539	3	.433	15	-30.768	4	0	4	-.022	4	-.001	4
159	4	max	940.728	2	1.682	4	.475	1	0	12	0	1	0	15
160		min	-1220.148	3	.405	15	-31.226	4	0	4	-.033	4	-.002	4
161	5	max	941.248	2	1.563	4	.475	1	0	12	0	1	0	15
162		min	-1219.758	3	.377	15	-31.684	4	0	4	-.044	4	-.003	4
163	6	max	941.769	2	1.444	4	.475	1	0	12	0	1	0	15
164		min	-1219.367	3	.349	15	-32.143	4	0	4	-.055	4	-.003	4
165	7	max	942.29	2	1.326	4	.475	1	0	12	0	1	0	15
166		min	-1218.977	3	.321	15	-32.601	4	0	4	-.067	4	-.004	4
167	8	max	942.81	2	1.207	4	.475	1	0	12	.001	1	0	15
168		min	-1218.586	3	.294	15	-33.06	4	0	4	-.078	4	-.004	4
169	9	max	943.331	2	1.088	4	.475	1	0	12	.001	1	-.001	15
170		min	-1218.196	3	.266	15	-33.518	4	0	4	-.09	4	-.004	4
171	10	max	943.852	2	.969	4	.475	1	0	12	.002	1	-.001	15
172		min	-1217.805	3	.221	12	-33.976	4	0	4	-.102	4	-.005	4
173	11	max	944.372	2	.85	4	.475	1	0	12	.002	1	-.001	15
174		min	-1217.415	3	.175	12	-34.435	4	0	4	-.115	4	-.005	4
175	12	max	944.893	2	.732	2	.475	1	0	12	.002	1	-.001	15
176		min	-1217.024	3	.128	12	-34.893	4	0	4	-.127	4	-.005	4
177	13	max	945.414	2	.639	2	.475	1	0	12	.002	1	-.001	15
178		min	-1216.634	3	.082	12	-35.351	4	0	4	-.139	4	-.006	4
179	14	max	945.934	2	.547	2	.475	1	0	12	.002	1	-.001	15
180		min	-1216.243	3	.028	3	-35.81	4	0	4	-.152	4	-.006	4
181	15	max	946.455	2	.454	2	.475	1	0	12	.002	1	-.001	15
182		min	-1215.853	3	-.041	3	-36.268	4	0	4	-.165	4	-.006	4
183	16	max	946.976	2	.361	2	.475	1	0	12	.003	1	-.001	15
184		min	-1215.462	3	-.11	3	-36.726	4	0	4	-.178	4	-.006	4
185	17	max	947.497	2	.269	2	.475	1	0	12	.003	1	-.002	15
186		min	-1215.072	3	-.18	3	-37.185	4	0	4	-.191	4	-.006	4
187	18	max	948.017	2	.176	2	.475	1	0	12	.003	1	-.001	12
188		min	-1214.681	3	-.249	3	-37.643	4	0	4	-.205	4	-.006	4
189	19	max	948.538	2	.084	2	.475	1	0	12	.003	1	-.001	12



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1214.291	3	-.319	3	-38.101	4	0	4	-.218	4	-.006	4
191	M3	1	max	707.376	2	7.679	4	7.766	4	0	12	0	1	.006	4
192			min	-854.879	3	1.814	15	.025	12	0	4	-.04	4	.001	12
193		2	max	707.206	2	6.918	4	8.3	4	0	12	0	1	.003	2
194			min	-855.006	3	1.635	15	.025	12	0	4	-.036	4	0	3
195		3	max	707.036	2	6.157	4	8.835	4	0	12	0	1	.001	2
196			min	-855.134	3	1.456	15	.025	12	0	4	-.033	4	-.001	3
197		4	max	706.865	2	5.397	4	9.37	4	0	12	.001	1	0	15
198			min	-855.262	3	1.277	15	.025	12	0	4	-.029	4	-.003	3
199		5	max	706.695	2	4.636	4	9.904	4	0	12	.001	1	0	15
200			min	-855.39	3	1.098	15	.025	12	0	4	-.025	4	-.004	6
201		6	max	706.524	2	3.875	4	10.439	4	0	12	.001	1	-.001	15
202			min	-855.517	3	.919	15	.025	12	0	4	-.021	5	-.006	6
203		7	max	706.354	2	3.114	4	10.974	4	0	12	.002	1	-.002	15
204			min	-855.645	3	.74	15	.025	12	0	4	-.017	5	-.007	6
205		8	max	706.184	2	2.353	4	11.508	4	0	12	.002	1	-.002	15
206			min	-855.773	3	.561	15	.025	12	0	4	-.012	5	-.008	6
207		9	max	706.013	2	1.592	4	12.043	4	0	12	.002	1	-.002	15
208			min	-855.901	3	.383	15	.025	12	0	4	-.007	5	-.009	6
209		10	max	705.843	2	.831	4	12.578	4	0	12	.002	1	-.002	15
210			min	-856.028	3	.189	12	.025	12	0	4	-.002	5	-.01	6
211		11	max	705.673	2	.193	2	13.112	4	0	12	.004	4	-.002	15
212			min	-856.156	3	-.182	3	.025	12	0	4	0	12	-.01	6
213		12	max	705.502	2	-.154	15	13.647	4	0	12	.009	4	-.002	15
214			min	-856.284	3	-.692	6	.025	12	0	4	0	12	-.01	6
215		13	max	705.332	2	-.333	15	14.182	4	0	12	.015	4	-.002	15
216			min	-856.412	3	-1.453	6	.025	12	0	4	0	12	-.009	6
217		14	max	705.162	2	-.512	15	14.717	4	0	12	.021	4	-.002	15
218			min	-856.539	3	-2.214	6	.025	12	0	4	0	12	-.009	6
219		15	max	704.991	2	-.691	15	15.251	4	0	12	.027	4	-.002	15
220			min	-856.667	3	-2.975	6	.025	12	0	4	0	12	-.007	6
221		16	max	704.821	2	-.87	15	15.786	4	0	12	.034	4	-.001	15
222			min	-856.795	3	-3.736	6	.025	12	0	4	0	12	-.006	6
223		17	max	704.651	2	-1.048	15	16.321	4	0	12	.04	4	-.001	15
224			min	-856.923	3	-4.497	6	.025	12	0	4	0	12	-.004	6
225		18	max	704.48	2	-1.227	15	16.855	4	0	12	.047	4	0	15
226			min	-857.051	3	-5.258	6	.025	12	0	4	0	12	-.002	6
227		19	max	704.31	2	-1.406	15	17.39	4	0	12	.054	4	0	1
228			min	-857.178	3	-6.019	6	.025	12	0	4	0	12	0	1
229	M4	1	max	992.377	1	0	1	-.961	12	0	1	.052	4	0	1
230			min	-38.534	5	0	1	-332.213	4	0	1	0	12	0	1
231		2	max	992.547	1	0	1	-.961	12	0	1	.014	4	0	1
232			min	-38.455	5	0	1	-332.361	4	0	1	0	12	0	1
233		3	max	992.717	1	0	1	-.961	12	0	1	0	12	0	1
234			min	-38.375	5	0	1	-332.508	4	0	1	-.025	4	0	1
235		4	max	992.888	1	0	1	-.961	12	0	1	0	12	0	1
236			min	-38.296	5	0	1	-332.656	4	0	1	-.063	4	0	1
237		5	max	993.058	1	0	1	-.961	12	0	1	0	12	0	1
238			min	-38.216	5	0	1	-332.803	4	0	1	-.101	4	0	1
239		6	max	993.228	1	0	1	-.961	12	0	1	0	12	0	1
240			min	-38.137	5	0	1	-332.951	4	0	1	-.139	4	0	1
241		7	max	993.399	1	0	1	-.961	12	0	1	0	12	0	1
242			min	-38.057	5	0	1	-333.099	4	0	1	-.178	4	0	1
243		8	max	993.569	1	0	1	-.961	12	0	1	0	12	0	1
244			min	-37.978	5	0	1	-333.246	4	0	1	-.216	4	0	1
245		9	max	993.739	1	0	1	-.961	12	0	1	0	12	0	1
246			min	-37.898	5	0	1	-333.394	4	0	1	-.254	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	993.91	1	0	1	-.961	12	0	1	0	12	0	1
248		min	-37.819	5	0	1	-333.542	4	0	1	-.292	4	0	1
249	11	max	994.08	1	0	1	-.961	12	0	1	0	12	0	1
250		min	-37.739	5	0	1	-333.689	4	0	1	-.331	4	0	1
251	12	max	994.251	1	0	1	-.961	12	0	1	0	12	0	1
252		min	-37.66	5	0	1	-333.837	4	0	1	-.369	4	0	1
253	13	max	994.421	1	0	1	-.961	12	0	1	-.001	12	0	1
254		min	-37.58	5	0	1	-333.984	4	0	1	-.407	4	0	1
255	14	max	994.591	1	0	1	-.961	12	0	1	-.001	12	0	1
256		min	-37.501	5	0	1	-334.132	4	0	1	-.446	4	0	1
257	15	max	994.762	1	0	1	-.961	12	0	1	-.001	12	0	1
258		min	-37.421	5	0	1	-334.28	4	0	1	-.484	4	0	1
259	16	max	994.932	1	0	1	-.961	12	0	1	-.001	12	0	1
260		min	-37.342	5	0	1	-334.427	4	0	1	-.522	4	0	1
261	17	max	995.102	1	0	1	-.961	12	0	1	-.002	12	0	1
262		min	-37.262	5	0	1	-334.575	4	0	1	-.561	4	0	1
263	18	max	995.273	1	0	1	-.961	12	0	1	-.002	12	0	1
264		min	-37.183	5	0	1	-334.723	4	0	1	-.599	4	0	1
265	19	max	995.443	1	0	1	-.961	12	0	1	-.002	12	0	1
266		min	-37.103	5	0	1	-334.87	4	0	1	-.638	4	0	1
267	M6	1	max	3039.415	2	2.213	2	0	1	0	0	4	0	1
268		min	-4014.361	3	.307	12	-30.186	4	0	4	0	1	0	1
269	2	max	3039.936	2	2.12	2	0	1	0	1	0	1	0	12
270		min	-4013.97	3	.26	12	-30.644	4	0	4	-.011	4	0	2
271	3	max	3040.457	2	2.027	2	0	1	0	1	0	1	0	12
272		min	-4013.579	3	.214	12	-31.103	4	0	4	-.022	4	-.002	2
273	4	max	3040.978	2	1.935	2	0	1	0	1	0	1	0	12
274		min	-4013.189	3	.168	12	-31.561	4	0	4	-.033	4	-.002	2
275	5	max	3041.498	2	1.842	2	0	1	0	1	0	1	0	12
276		min	-4012.798	3	.101	3	-32.019	4	0	4	-.044	4	-.003	2
277	6	max	3042.019	2	1.75	2	0	1	0	1	0	1	0	12
278		min	-4012.408	3	.032	3	-32.478	4	0	4	-.056	4	-.004	2
279	7	max	3042.54	2	1.657	2	0	1	0	1	0	1	0	12
280		min	-4012.017	3	-.038	3	-32.936	4	0	4	-.067	4	-.004	2
281	8	max	3043.06	2	1.564	2	0	1	0	1	0	1	0	3
282		min	-4011.627	3	-.107	3	-33.395	4	0	4	-.079	4	-.005	2
283	9	max	3043.581	2	1.472	2	0	1	0	1	0	1	0	3
284		min	-4011.236	3	-.177	3	-33.853	4	0	4	-.091	4	-.005	2
285	10	max	3044.102	2	1.379	2	0	1	0	1	0	1	0	3
286		min	-4010.846	3	-.246	3	-34.311	4	0	4	-.103	4	-.006	2
287	11	max	3044.622	2	1.287	2	0	1	0	1	0	1	0	3
288		min	-4010.455	3	-.315	3	-34.77	4	0	4	-.116	4	-.006	2
289	12	max	3045.143	2	1.194	2	0	1	0	1	0	1	0	3
290		min	-4010.065	3	-.385	3	-35.228	4	0	4	-.128	4	-.007	2
291	13	max	3045.664	2	1.101	2	0	1	0	1	0	1	0	3
292		min	-4009.674	3	-.454	3	-35.686	4	0	4	-.141	4	-.007	2
293	14	max	3046.184	2	1.009	2	0	1	0	1	0	1	0	3
294		min	-4009.284	3	-.524	3	-36.145	4	0	4	-.154	4	-.007	2
295	15	max	3046.705	2	.916	2	0	1	0	1	0	1	0	3
296		min	-4008.893	3	-.593	3	-36.603	4	0	4	-.167	4	-.008	2
297	16	max	3047.226	2	.823	2	0	1	0	1	0	1	0	3
298		min	-4008.503	3	-.663	3	-37.061	4	0	4	-.18	4	-.008	2
299	17	max	3047.747	2	.731	2	0	1	0	1	0	1	.001	3
300		min	-4008.112	3	-.732	3	-37.52	4	0	4	-.193	4	-.008	2
301	18	max	3048.267	2	.638	2	0	1	0	1	0	1	.001	3
302		min	-4007.722	3	-.802	3	-37.978	4	0	4	-.207	4	-.009	2
303	19	max	3048.788	2	.546	2	0	1	0	1	0	1	.002	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4007.331	3	-871	3	-38.436	4	0	4	-22	4	-009	2
305	M7	1	max	2668.792	2	7.694	6	7.311	4	0	1	0	1	.009	2
306			min	-2708.077	3	1.806	15	0	1	0	4	-.04	4	-.002	3
307		2	max	2668.622	2	6.933	6	7.846	4	0	1	0	1	.006	2
308			min	-2708.205	3	1.628	15	0	1	0	4	-.037	4	-.003	3
309		3	max	2668.452	2	6.172	6	8.38	4	0	1	0	1	.004	2
310			min	-2708.333	3	1.449	15	0	1	0	4	-.034	4	-.004	3
311		4	max	2668.281	2	5.411	6	8.915	4	0	1	0	1	.002	2
312			min	-2708.461	3	1.27	15	0	1	0	4	-.03	4	-.005	3
313		5	max	2668.111	2	4.65	6	9.45	4	0	1	0	1	0	2
314			min	-2708.588	3	1.091	15	0	1	0	4	-.026	4	-.006	3
315		6	max	2667.941	2	3.889	6	9.984	4	0	1	0	1	-.001	15
316			min	-2708.716	3	.912	15	0	1	0	4	-.022	4	-.007	3
317		7	max	2667.77	2	3.128	6	10.519	4	0	1	0	1	-.002	15
318			min	-2708.844	3	.733	15	0	1	0	4	-.018	4	-.008	3
319		8	max	2667.6	2	2.368	2	11.054	4	0	1	0	1	-.002	15
320			min	-2708.972	3	.499	12	0	1	0	4	-.013	4	-.008	4
321		9	max	2667.43	2	1.775	2	11.589	4	0	1	0	1	-.002	15
322			min	-2709.1	3	.202	12	0	1	0	4	-.009	4	-.009	4
323		10	max	2667.259	2	1.182	2	12.123	4	0	1	0	1	-.002	15
324			min	-2709.227	3	-.211	3	0	1	0	4	-.004	4	-.01	4
325		11	max	2667.089	2	.589	2	12.658	4	0	1	.001	4	-.002	15
326			min	-2709.355	3	-.656	3	0	1	0	4	0	1	-.01	4
327		12	max	2666.919	2	-.004	2	13.193	4	0	1	.007	4	-.002	15
328			min	-2709.483	3	-1.1	3	0	1	0	4	0	1	-.01	4
329		13	max	2666.748	2	-.34	15	13.727	4	0	1	.012	4	-.002	15
330			min	-2709.611	3	-1.545	3	0	1	0	4	0	1	-.009	4
331		14	max	2666.578	2	-.519	15	14.262	4	0	1	.018	4	-.002	15
332			min	-2709.738	3	-2.199	4	0	1	0	4	0	1	-.009	4
333		15	max	2666.407	2	-.698	15	14.797	4	0	1	.024	4	-.002	15
334			min	-2709.866	3	-2.96	4	0	1	0	4	0	1	-.007	4
335		16	max	2666.237	2	-.877	15	15.331	4	0	1	.031	4	-.001	15
336			min	-2709.994	3	-3.721	4	0	1	0	4	0	1	-.006	4
337		17	max	2666.067	2	-1.056	15	15.866	4	0	1	.037	4	-.001	15
338			min	-2710.122	3	-4.482	4	0	1	0	4	0	1	-.004	4
339		18	max	2665.896	2	-1.234	15	16.401	4	0	1	.044	4	0	15
340			min	-2710.249	3	-5.243	4	0	1	0	4	0	1	-.002	4
341		19	max	2665.726	2	-1.413	15	16.935	4	0	1	.051	4	0	1
342			min	-2710.377	3	-6.004	4	0	1	0	4	0	1	0	1
343	M8	1	max	2417.89	1	0	1	0	1	0	1	.048	4	0	1
344			min	105.273	15	0	1	-318.792	4	0	1	0	1	0	1
345		2	max	2418.061	1	0	1	0	1	0	1	.011	4	0	1
346			min	105.324	15	0	1	-318.939	4	0	1	0	1	0	1
347		3	max	2418.231	1	0	1	0	1	0	1	0	1	0	1
348			min	105.376	15	0	1	-319.087	4	0	1	-.025	4	0	1
349		4	max	2418.401	1	0	1	0	1	0	1	0	1	0	1
350			min	105.427	15	0	1	-319.235	4	0	1	-.062	4	0	1
351		5	max	2418.572	1	0	1	0	1	0	1	0	1	0	1
352			min	105.479	15	0	1	-319.382	4	0	1	-.098	4	0	1
353		6	max	2418.742	1	0	1	0	1	0	1	0	1	0	1
354			min	105.53	15	0	1	-319.53	4	0	1	-.135	4	0	1
355		7	max	2418.912	1	0	1	0	1	0	1	0	1	0	1
356			min	105.581	15	0	1	-319.677	4	0	1	-.172	4	0	1
357		8	max	2419.083	1	0	1	0	1	0	1	0	1	0	1
358			min	105.633	15	0	1	-319.825	4	0	1	-.209	4	0	1
359		9	max	2419.253	1	0	1	0	1	0	1	0	1	0	1
360			min	105.684	15	0	1	-319.973	4	0	1	-.245	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2419.423	1	0	1	0	1	0	1	0	1	0	1
362			min	105.735	15	0	1	-320.12	4	0	1	-.282	4	0	1
363		11	max	2419.594	1	0	1	0	1	0	1	0	1	0	1
364			min	105.787	15	0	1	-320.268	4	0	1	-.319	4	0	1
365		12	max	2419.764	1	0	1	0	1	0	1	0	1	0	1
366			min	105.838	15	0	1	-320.416	4	0	1	-.356	4	0	1
367		13	max	2419.934	1	0	1	0	1	0	1	0	1	0	1
368			min	105.89	15	0	1	-320.563	4	0	1	-.392	4	0	1
369		14	max	2420.105	1	0	1	0	1	0	1	0	1	0	1
370			min	105.941	15	0	1	-320.711	4	0	1	-.429	4	0	1
371		15	max	2420.275	1	0	1	0	1	0	1	0	1	0	1
372			min	105.992	15	0	1	-320.859	4	0	1	-.466	4	0	1
373		16	max	2420.445	1	0	1	0	1	0	1	0	1	0	1
374			min	106.044	15	0	1	-321.006	4	0	1	-.503	4	0	1
375		17	max	2420.616	1	0	1	0	1	0	1	0	1	0	1
376			min	106.095	15	0	1	-321.154	4	0	1	-.54	4	0	1
377		18	max	2420.786	1	0	1	0	1	0	1	0	1	0	1
378			min	106.147	15	0	1	-321.301	4	0	1	-.577	4	0	1
379		19	max	2420.957	1	0	1	0	1	0	1	0	1	0	1
380			min	106.198	15	0	1	-321.449	4	0	1	-.614	4	0	1
381	M10	1	max	939.165	2	1.995	6	-.027	12	0	1	0	2	0	1
382			min	-1221.32	3	.46	15	-30.172	4	0	5	0	3	0	1
383		2	max	939.686	2	1.876	6	-.027	12	0	1	0	10	0	15
384			min	-1220.929	3	.432	15	-30.631	4	0	5	-.011	4	0	6
385		3	max	940.207	2	1.757	6	-.027	12	0	1	0	12	0	15
386			min	-1220.539	3	.404	15	-31.089	4	0	5	-.022	4	-.001	6
387		4	max	940.728	2	1.638	6	-.027	12	0	1	0	12	0	15
388			min	-1220.148	3	.376	15	-31.547	4	0	5	-.033	4	-.002	6
389		5	max	941.248	2	1.52	6	-.027	12	0	1	0	12	0	15
390			min	-1219.758	3	.348	15	-32.006	4	0	5	-.044	4	-.003	6
391		6	max	941.769	2	1.401	6	-.027	12	0	1	0	12	0	15
392			min	-1219.367	3	.32	15	-32.464	4	0	5	-.056	4	-.003	6
393		7	max	942.29	2	1.282	6	-.027	12	0	1	0	12	0	15
394			min	-1218.977	3	.292	15	-32.923	4	0	5	-.067	4	-.004	6
395		8	max	942.81	2	1.163	6	-.027	12	0	1	0	12	0	15
396			min	-1218.586	3	.264	15	-33.381	4	0	5	-.079	4	-.004	6
397		9	max	943.331	2	1.044	6	-.027	12	0	1	0	12	0	15
398			min	-1218.196	3	.236	15	-33.839	4	0	5	-.091	4	-.004	6
399		10	max	943.852	2	.925	6	-.027	12	0	1	0	12	-.001	15
400			min	-1217.805	3	.208	15	-34.298	4	0	5	-.103	4	-.005	6
401		11	max	944.372	2	.825	2	-.027	12	0	1	0	12	-.001	15
402			min	-1217.415	3	.175	12	-34.756	4	0	5	-.116	4	-.005	6
403		12	max	944.893	2	.732	2	-.027	12	0	1	0	12	-.001	15
404			min	-1217.024	3	.128	12	-35.214	4	0	5	-.128	4	-.005	6
405		13	max	945.414	2	.639	2	-.027	12	0	1	0	12	-.001	15
406			min	-1216.634	3	.082	12	-35.673	4	0	5	-.141	4	-.005	6
407		14	max	945.934	2	.547	2	-.027	12	0	1	0	12	-.001	15
408			min	-1216.243	3	.028	3	-36.131	4	0	5	-.154	4	-.006	6
409		15	max	946.455	2	.454	2	-.027	12	0	1	0	12	-.001	15
410			min	-1215.853	3	-.041	3	-36.589	4	0	5	-.167	4	-.006	6
411		16	max	946.976	2	.361	2	-.027	12	0	1	0	12	-.001	15
412			min	-1215.462	3	-.11	3	-37.048	4	0	5	-.18	4	-.006	6
413		17	max	947.497	2	.269	2	-.027	12	0	1	0	12	-.001	15
414			min	-1215.072	3	-.18	3	-37.506	4	0	5	-.193	4	-.006	6
415		18	max	948.017	2	.176	2	-.027	12	0	1	0	12	-.001	15
416			min	-1214.681	3	-.249	3	-37.964	4	0	5	-.206	4	-.006	6
417		19	max	948.538	2	.084	2	-.027	12	0	1	0	12	-.001	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1214.291	3	-.319	3	-38.423	4	0	5	-.22	4	-.006	6
419	M11	1	max	707.376	2	7.642	6	7.494	4	0	1	0	12	.006	6
420			min	-854.879	3	1.788	15	-.401	1	0	4	-.04	4	.001	15
421		2	max	707.206	2	6.881	6	8.029	4	0	1	0	12	.003	2
422			min	-855.006	3	1.61	15	-.401	1	0	4	-.037	4	0	3
423		3	max	707.036	2	6.12	6	8.564	4	0	1	0	12	.001	2
424			min	-855.134	3	1.431	15	-.401	1	0	4	-.033	4	-.001	3
425		4	max	706.865	2	5.359	6	9.098	4	0	1	0	12	0	15
426			min	-855.262	3	1.252	15	-.401	1	0	4	-.03	4	-.003	3
427		5	max	706.695	2	4.598	6	9.633	4	0	1	0	12	-.001	15
428			min	-855.39	3	1.073	15	-.401	1	0	4	-.026	4	-.004	4
429		6	max	706.524	2	3.837	6	10.168	4	0	1	0	12	-.001	15
430			min	-855.517	3	.894	15	-.401	1	0	4	-.022	4	-.006	4
431		7	max	706.354	2	3.076	6	10.702	4	0	1	0	12	-.002	15
432			min	-855.645	3	.715	15	-.401	1	0	4	-.017	4	-.007	4
433		8	max	706.184	2	2.315	6	11.237	4	0	1	0	12	-.002	15
434			min	-855.773	3	.536	15	-.401	1	0	4	-.013	4	-.009	4
435		9	max	706.013	2	1.554	6	11.772	4	0	1	0	12	-.002	15
436			min	-855.901	3	.357	15	-.401	1	0	4	-.008	4	-.009	4
437		10	max	705.843	2	.793	6	12.306	4	0	1	0	12	-.002	15
438			min	-856.028	3	.178	15	-.401	1	0	4	-.003	4	-.01	4
439		11	max	705.673	2	.193	2	12.841	4	0	1	.003	5	-.002	15
440			min	-856.156	3	-.182	3	-.401	1	0	4	-.002	1	-.01	4
441		12	max	705.502	2	-.179	15	13.376	4	0	1	.008	5	-.002	15
442			min	-856.284	3	-.73	4	-.401	1	0	4	-.002	1	-.01	4
443		13	max	705.332	2	-.358	15	13.91	4	0	1	.014	5	-.002	15
444			min	-856.412	3	-1.49	4	-.401	1	0	4	-.003	1	-.009	4
445		14	max	705.162	2	-.537	15	14.445	4	0	1	.02	5	-.002	15
446			min	-856.539	3	-2.251	4	-.401	1	0	4	-.003	1	-.009	4
447		15	max	704.991	2	-.716	15	14.98	4	0	1	.026	5	-.002	15
448			min	-856.667	3	-3.012	4	-.401	1	0	4	-.003	1	-.008	4
449		16	max	704.821	2	-.895	15	15.514	4	0	1	.032	5	-.001	15
450			min	-856.795	3	-3.773	4	-.401	1	0	4	-.003	1	-.006	4
451		17	max	704.651	2	-1.074	15	16.049	4	0	1	.039	5	-.001	15
452			min	-856.923	3	-4.534	4	-.401	1	0	4	-.003	1	-.004	4
453		18	max	704.48	2	-1.253	15	16.584	4	0	1	.045	5	0	15
454			min	-857.051	3	-5.295	4	-.401	1	0	4	-.003	1	-.002	4
455		19	max	704.31	2	-1.431	15	17.119	4	0	1	.052	5	0	1
456			min	-857.178	3	-6.056	4	-.401	1	0	4	-.004	1	0	1
457	M12	1	max	992.377	1	0	1	15.525	1	0	1	.05	5	0	1
458			min	69.382	12	0	1	-321.67	4	0	1	-.003	1	0	1
459		2	max	992.547	1	0	1	15.525	1	0	1	.013	5	0	1
460			min	69.467	12	0	1	-321.818	4	0	1	-.002	1	0	1
461		3	max	992.717	1	0	1	15.525	1	0	1	0	1	0	1
462			min	69.553	12	0	1	-321.965	4	0	1	-.025	4	0	1
463		4	max	992.888	1	0	1	15.525	1	0	1	.002	1	0	1
464			min	69.638	12	0	1	-322.113	4	0	1	-.061	4	0	1
465		5	max	993.058	1	0	1	15.525	1	0	1	.004	1	0	1
466			min	69.723	12	0	1	-322.261	4	0	1	-.098	4	0	1
467		6	max	993.228	1	0	1	15.525	1	0	1	.006	1	0	1
468			min	69.808	12	0	1	-322.408	4	0	1	-.136	4	0	1
469		7	max	993.399	1	0	1	15.525	1	0	1	.007	1	0	1
470			min	69.893	12	0	1	-322.556	4	0	1	-.173	4	0	1
471		8	max	993.569	1	0	1	15.525	1	0	1	.009	1	0	1
472			min	69.978	12	0	1	-322.704	4	0	1	-.21	4	0	1
473		9	max	993.739	1	0	1	15.525	1	0	1	.011	1	0	1
474			min	70.064	12	0	1	-322.851	4	0	1	-.247	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	993.91	1	0	1	15.525	1	0	1	.013	1	0	1
476			min	70.149	12	0	1	-322.999	4	0	1	-.284	4	0	1
477		11	max	994.08	1	0	1	15.525	1	0	1	.014	1	0	1
478			min	70.234	12	0	1	-323.147	4	0	1	-.321	4	0	1
479		12	max	994.251	1	0	1	15.525	1	0	1	.016	1	0	1
480			min	70.319	12	0	1	-323.294	4	0	1	-.358	4	0	1
481		13	max	994.421	1	0	1	15.525	1	0	1	.018	1	0	1
482			min	70.404	12	0	1	-323.442	4	0	1	-.395	4	0	1
483		14	max	994.591	1	0	1	15.525	1	0	1	.02	1	0	1
484			min	70.489	12	0	1	-323.589	4	0	1	-.432	4	0	1
485		15	max	994.762	1	0	1	15.525	1	0	1	.022	1	0	1
486			min	70.575	12	0	1	-323.737	4	0	1	-.469	4	0	1
487		16	max	994.932	1	0	1	15.525	1	0	1	.023	1	0	1
488			min	70.66	12	0	1	-323.885	4	0	1	-.507	4	0	1
489		17	max	995.102	1	0	1	15.525	1	0	1	.025	1	0	1
490			min	70.745	12	0	1	-324.032	4	0	1	-.544	4	0	1
491		18	max	995.273	1	0	1	15.525	1	0	1	.027	1	0	1
492			min	70.83	12	0	1	-324.18	4	0	1	-.581	4	0	1
493		19	max	995.443	1	0	1	15.525	1	0	1	.029	1	0	1
494			min	70.915	12	0	1	-324.328	4	0	1	-.618	4	0	1
495	M1	1	max	201.014	1	640.972	3	46.565	5	0	2	.336	1	.002	3
496			min	-7.995	5	-386.784	2	-140.069	1	0	3	-.118	5	-.012	2
497		2	max	201.835	1	640.092	3	47.807	5	0	2	.262	1	.192	2
498			min	-7.612	5	-387.958	2	-140.069	1	0	3	-.093	5	-.336	3
499		3	max	533.99	3	459.891	2	20.024	5	0	3	.188	1	.387	2
500			min	-304.891	2	-471.152	3	-139.862	1	0	2	-.068	5	-.66	3
501		4	max	534.607	3	458.718	2	21.265	5	0	3	.114	1	.149	1
502			min	-304.069	2	-472.032	3	-139.862	1	0	2	-.057	5	-.412	3
503		5	max	535.223	3	457.545	2	22.507	5	0	3	.041	1	-.003	15
504			min	-303.248	2	-472.912	3	-139.862	1	0	2	-.045	5	-.162	3
505		6	max	535.839	3	456.371	2	23.748	5	0	3	-.002	12	.088	3
506			min	-302.426	2	-473.792	3	-139.862	1	0	2	-.041	4	-.338	2
507		7	max	536.455	3	455.198	2	24.99	5	0	3	-.007	12	.338	3
508			min	-301.605	2	-474.672	3	-139.862	1	0	2	-.107	1	-.579	2
509		8	max	537.071	3	454.024	2	26.231	5	0	3	-.004	15	.589	3
510			min	-300.783	2	-475.552	3	-139.862	1	0	2	-.181	1	-.819	2
511		9	max	555.761	3	49.19	2	70.08	5	0	9	.104	1	.686	3
512			min	-209.887	2	.355	15	-200.479	1	0	3	-.156	5	-.939	2
513		10	max	556.377	3	48.017	2	71.321	5	0	9	0	12	.669	3
514			min	-209.066	2	.001	15	-200.479	1	0	3	-.12	4	-.964	2
515		11	max	556.993	3	46.844	2	72.563	5	0	9	-.007	12	.652	3
516			min	-208.244	2	-1.439	4	-200.479	1	0	3	-.108	4	-.989	2
517		12	max	575.608	3	319.648	3	179.381	5	0	2	.178	1	.569	3
518			min	-117.336	2	-555.34	2	-136.668	1	0	3	-.248	5	-.878	2
519		13	max	576.224	3	318.768	3	180.622	5	0	2	.106	1	.4	3
520			min	-116.515	2	-556.514	2	-136.668	1	0	3	-.153	5	-.585	2
521		14	max	576.84	3	317.888	3	181.863	5	0	2	.034	1	.232	3
522			min	-115.693	2	-557.687	2	-136.668	1	0	3	-.057	5	-.291	2
523		15	max	577.456	3	317.008	3	183.105	5	0	2	.039	5	.065	3
524			min	-114.871	2	-558.861	2	-136.668	1	0	3	-.038	1	-.019	9
525		16	max	578.073	3	316.128	3	184.346	5	0	2	.136	5	.299	2
526			min	-114.05	2	-560.034	2	-136.668	1	0	3	-.11	1	-.102	3
527		17	max	578.689	3	315.248	3	185.588	5	0	2	.234	5	.595	2
528			min	-113.228	2	-561.207	2	-136.668	1	0	3	-.182	1	-.269	3
529		18	max	17.29	5	585.325	2	-9.421	12	0	3	.236	5	.3	2
530			min	-202.062	1	-270.69	3	-158.635	4	0	2	-.258	1	-.133	3
531		19	max	17.673	5	584.151	2	-9.421	12	0	3	.174	5	.01	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532	M5	min	-201.24	1	-271.571	3	-157.394	4	0	2	-.338	1	-.009	2
533		max	431.617	1	2136.825	3	114.023	5	0	1	0	1	.024	2
534		min	22.723	12	-1306.471	2	0	1	0	4	-.279	4	-.004	3
535		max	432.438	1	2135.945	3	115.265	5	0	1	0	1	.714	2
536		min	23.134	12	-1307.645	2	0	1	0	4	-.219	4	-1.131	3
537		max	1724.414	3	1416.89	2	91.097	4	0	4	0	1	1.372	2
538		min	-1087.494	2	-1536.078	3	0	1	0	1	-.158	4	-2.214	3
539		max	1725.03	3	1415.717	2	92.338	4	0	4	0	1	.624	2
540		min	-1086.672	2	-1536.958	3	0	1	0	1	-.109	4	-1.403	3
541		max	1725.646	3	1414.543	2	93.579	4	0	4	0	1	0	9
542	M6	min	-1085.85	2	-1537.838	3	0	1	0	1	-.06	4	-.592	3
543		max	1726.262	3	1413.37	2	94.821	4	0	4	0	1	.22	3
544		min	-1085.029	2	-1538.718	3	0	1	0	1	-.011	4	-.869	2
545		max	1726.878	3	1412.196	2	96.062	4	0	4	.04	4	1.032	3
546		min	-1084.207	2	-1539.598	3	0	1	0	1	0	1	-1.614	2
547		max	1727.495	3	1411.023	2	97.304	4	0	4	.091	4	1.845	3
548		min	-1083.386	2	-1540.478	3	0	1	0	1	0	1	-2.359	2
549		max	1761.122	3	164.244	2	235.209	4	0	1	0	1	2.118	3
550		min	-897.111	2	.357	15	0	1	0	1	-.236	4	-2.69	2
551		max	1761.738	3	163.07	2	236.451	4	0	1	0	1	2.059	3
552	M7	min	-896.289	2	.003	15	0	1	0	1	-.111	4	-2.776	2
553		max	1762.354	3	161.897	2	237.692	4	0	1	.014	4	.2	3
554		min	-895.468	2	-1.232	6	0	1	0	1	0	1	-2.862	2
555		max	1796.13	3	1036.44	3	269.746	4	0	1	0	1	1.76	3
556		min	-709.217	2	-1745.211	2	0	1	0	4	-.374	4	-2.565	2
557		max	1796.746	3	1035.56	3	270.987	4	0	1	0	1	1.213	3
558		min	-708.396	2	-1746.385	2	0	1	0	4	-.231	4	-1.644	2
559		max	1797.363	3	1034.68	3	272.229	4	0	1	0	1	.667	3
560		min	-707.574	2	-1747.558	2	0	1	0	4	-.088	4	-.722	2
561		max	1797.979	3	1033.8	3	273.47	4	0	1	.056	4	.2	2
562	M8	min	-706.752	2	-1748.731	2	0	1	0	4	0	1	-.004	13
563		max	1798.595	3	1032.92	3	274.712	4	0	1	.201	4	1.123	2
564		min	-705.931	2	-1749.905	2	0	1	0	4	0	1	-.424	3
565		max	1799.211	3	1032.04	3	275.953	4	0	1	.346	4	2.047	2
566		min	-705.109	2	-1751.078	2	0	1	0	4	0	1	-.969	3
567		max	-23.426	12	1965.327	2	0	1	0	4	.397	4	1.054	2
568		min	-431.985	1	-921.46	3	-22.686	5	0	1	0	1	-.506	3
569		max	-23.016	12	1964.154	2	0	1	0	4	.386	4	.018	2
570		min	-431.164	1	-922.34	3	-21.445	5	0	1	0	1	-.02	3
571		max	201.014	1	640.972	3	140.069	1	0	3	-.022	12	.002	3
572	M9	min	11.691	12	-386.784	2	9.028	12	0	4	-.336	1	-.012	2
573		max	201.835	1	640.092	3	140.069	1	0	3	-.017	12	.192	2
574		min	12.102	12	-387.958	2	9.028	12	0	4	-.262	1	-.336	3
575		max	533.99	3	459.891	2	139.862	1	0	2	-.012	12	.387	2
576		min	-304.891	2	-471.152	3	9.003	12	0	3	-.188	1	-.66	3
577		max	534.607	3	458.718	2	139.862	1	0	2	-.007	12	.149	1
578		min	-304.069	2	-472.032	3	9.003	12	0	3	-.114	1	-.412	3
579		max	535.223	3	457.545	2	139.862	1	0	2	-.003	12	-.003	15
580		min	-303.248	2	-472.912	3	9.003	12	0	3	-.063	4	-.162	3
581		max	535.839	3	456.371	2	139.862	1	0	2	.033	1	.088	3
582	M10	min	-302.426	2	-473.792	3	9.003	12	0	3	-.027	5	-.338	2
583		max	536.455	3	455.198	2	139.862	1	0	2	.107	1	.338	3
584		min	-301.605	2	-474.672	3	9.003	12	0	3	-.002	5	-.579	2
585		max	537.071	3	454.024	2	139.862	1	0	2	.181	1	.589	3
586		min	-300.783	2	-475.552	3	9.003	12	0	3	.012	12	-.819	2
587		max	555.761	3	49.19	2	200.479	1	0	3	-.007	12	.686	3
588		min	-209.887	2	.363	15	12.68	12	0	9	-.202	4	-.939	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	556.377	3	48.017	2	200.479	1	0	3	.001	1	.669	3
590		min	-209.066	2	.009	15	12.68	12	0	9	-.119	4	-.964	2
591	11	max	556.993	3	46.844	2	200.479	1	0	3	.107	1	.652	3
592		min	-208.244	2	-1.389	6	12.68	12	0	9	-.063	5	-.989	2
593	12	max	575.608	3	319.648	3	240.307	4	0	3	-.011	12	.569	3
594		min	-117.336	2	-555.34	2	8.49	12	0	2	-.327	4	-.878	2
595	13	max	576.224	3	318.768	3	241.548	4	0	3	-.007	12	.4	3
596		min	-116.515	2	-556.514	2	8.49	12	0	2	-.2	4	-.585	2
597	14	max	576.84	3	317.888	3	242.79	4	0	3	-.002	12	.232	3
598		min	-115.693	2	-557.687	2	8.49	12	0	2	-.072	4	-.291	2
599	15	max	577.456	3	317.008	3	244.031	4	0	3	.056	4	.065	3
600		min	-114.871	2	-558.861	2	8.49	12	0	2	.002	12	-.019	9
601	16	max	578.073	3	316.128	3	245.273	4	0	3	.185	4	.299	2
602		min	-114.05	2	-560.034	2	8.49	12	0	2	.007	12	-.102	3
603	17	max	578.689	3	315.248	3	246.514	4	0	3	.315	4	.595	2
604		min	-113.228	2	-561.207	2	8.49	12	0	2	.011	12	-.269	3
605	18	max	-11.955	12	585.325	2	151.47	1	0	2	.35	4	.3	2
606		min	-202.062	1	-270.69	3	-93.376	5	0	3	.016	12	-.133	3
607	19	max	-11.544	12	584.151	2	151.47	1	0	2	.338	1	.01	3
608		min	-201.24	1	-271.571	3	-92.135	5	0	3	.021	12	-.009	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.095	2	.009	3	8.048e-3	2	NC	1	NC	1
2			min	-.859	4	-.014	3	-.005	2	-1.689e-3	3	NC	1	NC	1
3		2	max	.001	1	.416	3	.063	1	9.388e-3	2	NC	5	NC	2
4			min	-.859	4	-.148	1	-.034	5	-1.887e-3	3	626.845	3	4390.743	1
5		3	max	.001	1	.765	3	.154	1	1.073e-2	2	NC	5	NC	3
6			min	-.859	4	-.337	2	-.039	5	-2.086e-3	3	346.473	3	1770.464	1
7		4	max	0	1	.976	3	.233	1	1.207e-2	2	NC	15	NC	3
8			min	-.859	4	-.445	2	-.024	5	-2.284e-3	3	272.598	3	1163.468	1
9		5	max	0	1	1.024	3	.275	1	1.341e-2	2	NC	15	NC	5
10			min	-.859	4	-1.455	2	0	15	-2.482e-3	3	259.98	3	986.696	1
11		6	max	0	1	.913	3	.266	1	1.475e-2	2	NC	5	NC	5
12			min	-.859	4	-.371	2	.017	15	-2.681e-3	3	291.205	3	1018.438	1
13		7	max	0	1	.675	3	.21	1	1.609e-2	2	NC	5	NC	10
14			min	-.859	4	-.22	1	.023	10	-2.879e-3	3	391.573	3	1293.194	1
15		8	max	0	1	.374	3	.123	1	1.743e-2	2	NC	5	NC	10
16			min	-.859	4	-.037	1	.008	10	-3.077e-3	3	695.626	3	2224.226	1
17		9	max	0	1	.156	2	.048	4	1.877e-2	2	NC	4	NC	2
18			min	-.859	4	.004	15	-.006	10	-3.276e-3	3	2347.185	3	5615.026	4
19		10	max	0	1	.234	2	.029	3	2.011e-2	2	NC	3	NC	1
20			min	-.859	4	-.023	3	-.02	2	-3.474e-3	3	1941.621	2	NC	1
21		11	max	0	12	.156	2	.035	1	1.877e-2	2	NC	4	NC	2
22			min	-.859	4	.004	15	-.028	5	-3.276e-3	3	2347.185	3	7922.599	1
23		12	max	0	12	.374	3	.123	1	1.743e-2	2	NC	5	NC	4
24			min	-.859	4	-.037	1	-.027	5	-3.077e-3	3	695.626	3	2224.226	1
25		13	max	0	12	.675	3	.21	1	1.609e-2	2	NC	5	NC	5
26			min	-.86	4	-.22	1	-.007	5	-2.879e-3	3	391.573	3	1293.194	1
27		14	max	0	12	.913	3	.266	1	1.475e-2	2	NC	5	NC	5
28			min	-.86	4	-.371	2	.014	15	-2.681e-3	3	291.205	3	1018.438	1
29		15	max	0	12	1.024	3	.275	1	1.341e-2	2	NC	15	NC	12
30			min	-.86	4	-.455	2	.026	12	-2.482e-3	3	259.98	3	986.696	1
31		16	max	0	12	.976	3	.233	1	1.207e-2	2	NC	15	NC	3
32			min	-.86	4	-.445	2	.022	12	-2.284e-3	3	272.598	3	1163.468	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.765	3	.154	1	1.073e-2	2	NC	5	NC	3
34		min	-.86	4	-.337	2	.016	12	-2.086e-3	3	346.473	3	1770.464	1
35	18	max	0	12	.416	3	.063	4	9.388e-3	2	NC	5	NC	2
36		min	-.86	4	-.148	1	.006	10	-1.887e-3	3	626.845	3	4275.736	4
37	19	max	0	12	.095	2	.009	3	8.048e-3	2	NC	1	NC	1
38		min	-.86	4	-.014	3	-.005	2	-1.689e-3	3	NC	1	NC	1
39	M14	1	max	0	.209	3	.008	3	4.747e-3	2	NC	1	NC	1
40		min	-.623	4	-.316	2	-.004	2	-3.587e-3	3	NC	1	NC	1
41	2	max	0	1	.613	3	.044	1	5.736e-3	2	NC	5	NC	2
42		min	-.623	4	-.676	2	-.05	5	-4.411e-3	3	667.723	3	5229.243	5
43	3	max	0	1	.952	3	.125	1	6.725e-3	2	NC	15	NC	3
44		min	-.624	4	-.982	2	-.058	5	-5.235e-3	3	363.125	3	2176.227	1
45	4	max	0	1	1.182	3	.202	1	7.714e-3	2	NC	15	NC	3
46		min	-.624	4	-1.201	2	-.037	5	-6.059e-3	3	277.436	3	1344.404	1
47	5	max	0	1	1.281	3	.246	1	8.703e-3	2	9173.874	15	NC	5
48		min	-.624	4	-1.312	2	0	15	-6.883e-3	3	251.943	3	1102.604	1
49	6	max	0	1	1.249	3	.244	1	9.692e-3	2	9253.894	15	NC	12
50		min	-.624	4	-1.317	2	.025	12	-7.707e-3	3	259.611	3	1114.244	1
51	7	max	0	1	1.111	3	.195	1	1.068e-2	2	NC	15	NC	10
52		min	-.624	4	-1.233	2	.021	10	-8.531e-3	3	294.414	2	1393.882	1
53	8	max	0	1	.913	3	.115	1	1.167e-2	2	NC	15	NC	3
54		min	-.624	4	-1.096	2	.008	10	-9.355e-3	3	346.323	2	2368.343	1
55	9	max	0	1	.724	3	.068	4	1.266e-2	2	NC	5	NC	2
56		min	-.624	4	-.959	2	-.005	10	-1.018e-2	3	420.146	2	4056.101	4
57	10	max	0	1	.637	3	.026	3	1.365e-2	2	NC	5	NC	1
58		min	-.624	4	-.894	2	-.018	2	-1.1e-2	3	467.285	2	NC	1
59	11	max	0	12	.724	3	.034	1	1.266e-2	2	NC	5	NC	2
60		min	-.624	4	-.959	2	-.049	5	-1.018e-2	3	420.146	2	5512.223	5
61	12	max	0	12	.913	3	.115	1	1.167e-2	2	NC	15	NC	3
62		min	-.624	4	-1.096	2	-.054	5	-9.355e-3	3	346.323	2	2368.343	1
63	13	max	0	12	1.111	3	.195	1	1.068e-2	2	NC	15	NC	4
64		min	-.624	4	-1.233	2	-.032	5	-8.531e-3	3	294.414	2	1393.882	1
65	14	max	0	12	1.249	3	.244	1	9.692e-3	2	9253.528	15	NC	5
66		min	-.624	4	-1.317	2	.003	15	-7.707e-3	3	259.611	3	1114.244	1
67	15	max	0	12	1.281	3	.246	1	8.703e-3	2	9173.424	15	NC	12
68		min	-.624	4	-1.312	2	.023	12	-6.883e-3	3	251.943	3	1102.604	1
69	16	max	0	12	1.182	3	.202	1	7.714e-3	2	NC	15	NC	3
70		min	-.624	4	-1.201	2	.019	12	-6.059e-3	3	277.436	3	1344.404	1
71	17	max	0	12	.952	3	.125	1	6.725e-3	2	NC	15	NC	3
72		min	-.624	4	-.982	2	.013	12	-5.235e-3	3	363.125	3	2176.227	1
73	18	max	0	12	.613	3	.071	4	5.736e-3	2	NC	5	NC	2
74		min	-.624	4	-.676	2	.003	10	-4.411e-3	3	667.723	3	3810.557	4
75	19	max	0	12	.209	3	.008	3	4.747e-3	2	NC	1	NC	1
76		min	-.624	4	-.316	2	-.004	2	-3.587e-3	3	NC	1	NC	1
77	M15	1	max	0	.212	3	.008	3	3.171e-3	3	NC	1	NC	1
78		min	-.498	4	-.315	2	-.004	2	-4.991e-3	2	NC	1	NC	1
79	2	max	0	12	.469	3	.044	1	3.907e-3	3	NC	5	NC	2
80		min	-.498	4	-.806	2	-.064	5	-6.036e-3	2	549.936	2	4148.584	5
81	3	max	0	12	.689	3	.126	1	4.643e-3	3	NC	15	NC	3
82		min	-.498	4	-1.219	2	-.075	5	-7.081e-3	2	298.557	2	2170.596	1
83	4	max	0	12	.847	3	.202	1	5.379e-3	3	NC	15	NC	3
84		min	-.498	4	-1.502	2	-.05	5	-8.125e-3	2	227.414	2	1341.716	1
85	5	max	0	12	.929	3	.247	1	6.115e-3	3	9189.649	15	NC	5
86		min	-.498	4	-1.629	2	-.007	5	-9.17e-3	2	205.52	2	1100.615	1
87	6	max	0	12	.936	3	.244	1	6.851e-3	3	9273.194	15	NC	12
88		min	-.498	4	-1.6	2	.024	12	-1.021e-2	2	210.156	2	1112.166	1
89	7	max	0	12	.88	3	.195	1	7.587e-3	3	NC	15	NC	10





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-498	4	-1.443	2	.022	10	-1.126e-2	2	239.313	2	1390.7	1
91	8	max	0	12	.786	3	.123	4	8.323e-3	3	NC	15	NC	3
92		min	-498	4	-1.214	2	.009	10	-1.23e-2	2	300.34	2	2222.068	4
93	9	max	0	12	.691	3	.081	4	9.06e-3	3	NC	5	NC	2
94		min	-498	4	-.994	2	-.004	10	-1.335e-2	2	397.763	2	3421.871	4
95	10	max	0	1	.646	3	.024	3	9.796e-3	3	NC	5	NC	1
96		min	-498	4	-.891	2	-.017	2	-1.439e-2	2	468.446	2	NC	1
97	11	max	0	1	.691	3	.034	1	9.06e-3	3	NC	5	NC	2
98		min	-498	4	-.994	2	-.061	5	-1.335e-2	2	397.763	2	4408.539	5
99	12	max	0	1	.786	3	.116	1	8.323e-3	3	NC	15	NC	3
100		min	-498	4	-1.214	2	-.069	5	-1.23e-2	2	300.34	2	2359.576	1
101	13	max	0	1	.88	3	.195	1	7.587e-3	3	NC	15	NC	4
102		min	-498	4	-1.443	2	-.043	5	-1.126e-2	2	239.313	2	1390.7	1
103	14	max	0	1	.936	3	.244	1	6.851e-3	3	9272.909	15	NC	5
104		min	-498	4	-1.6	2	0	15	-1.021e-2	2	210.156	2	1112.166	1
105	15	max	0	1	.929	3	.247	1	6.115e-3	3	9189.302	15	NC	12
106		min	-498	4	-1.629	2	.022	12	-9.17e-3	2	205.52	2	1100.615	1
107	16	max	0	1	.847	3	.202	1	5.379e-3	3	NC	15	NC	3
108		min	-498	4	-1.502	2	.018	12	-8.125e-3	2	227.414	2	1341.716	1
109	17	max	0	1	.689	3	.131	4	4.643e-3	3	NC	15	NC	3
110		min	-498	4	-1.219	2	.013	12	-7.081e-3	2	298.557	2	2066.002	4
111	18	max	0	1	.469	3	.085	4	3.907e-3	3	NC	5	NC	2
112		min	-498	4	-.806	2	.003	10	-6.036e-3	2	549.936	2	3195.079	4
113	19	max	0	1	.212	3	.008	3	3.171e-3	3	NC	1	NC	1
114		min	-497	4	-.315	2	-.004	2	-4.991e-3	2	NC	1	NC	1
115	M16	1	max	0	.084	2	.007	3	5.439e-3	3	NC	1	NC	1
116		min	-.15	4	-.067	3	-.004	2	-6.53e-3	2	NC	1	NC	1
117	2	max	0	12	.098	3	.062	1	6.522e-3	3	NC	5	NC	2
118		min	-.15	4	-.293	2	-.05	5	-7.505e-3	2	717.661	2	4422.173	1
119	3	max	0	12	.229	3	.153	1	7.604e-3	3	NC	5	NC	3
120		min	-.15	4	-.594	2	-.06	5	-8.48e-3	2	398.401	2	1776.773	1
121	4	max	0	12	.301	3	.233	1	8.687e-3	3	NC	5	NC	3
122		min	-.15	4	-.771	2	-.042	5	-9.455e-3	2	316.008	2	1165.418	1
123	5	max	0	12	.305	3	.274	1	9.769e-3	3	NC	15	NC	5
124		min	-.15	4	-.8	2	-.009	5	-1.043e-2	2	305.704	2	986.887	1
125	6	max	0	12	.242	3	.266	1	1.085e-2	3	NC	5	NC	5
126		min	-.15	4	-.684	2	.017	15	-1.141e-2	2	351.6	2	1016.942	1
127	7	max	0	12	.128	3	.211	1	1.193e-2	3	NC	5	NC	12
128		min	-.15	4	-.456	2	.022	12	-1.238e-2	2	500.655	2	1287.856	1
129	8	max	0	12	0	15	.124	1	1.302e-2	3	NC	4	NC	3
130		min	-.15	4	-.171	2	.011	10	-1.336e-2	2	1058.753	2	2200.694	1
131	9	max	0	12	.098	1	.061	4	1.41e-2	3	NC	1	NC	2
132		min	-.15	4	-.133	3	-.003	10	-1.433e-2	2	4060.733	3	4563.328	4
133	10	max	0	1	.199	2	.02	3	1.518e-2	3	NC	4	NC	1
134		min	-.15	4	-.188	3	-.016	2	-1.531e-2	2	2222.997	3	NC	1
135	11	max	0	1	.098	1	.037	1	1.41e-2	3	NC	1	NC	2
136		min	-.15	4	-.133	3	-.041	5	-1.433e-2	2	4060.733	3	6605.128	5
137	12	max	0	1	-.001	15	.124	1	1.302e-2	3	NC	4	NC	3
138		min	-.15	4	-.171	2	-.041	5	-1.336e-2	2	1058.753	2	2200.694	1
139	13	max	0	1	.128	3	.211	1	1.193e-2	3	NC	5	NC	5
140		min	-.15	4	-.456	2	-.016	5	-1.238e-2	2	500.655	2	1287.856	1
141	14	max	0	1	.242	3	.266	1	1.085e-2	3	NC	5	NC	5
142		min	-.15	4	-.684	2	.013	15	-1.141e-2	2	351.6	2	1016.942	1
143	15	max	0	1	.305	3	.274	1	9.769e-3	3	NC	15	NC	12
144		min	-.149	4	-.8	2	.023	12	-1.043e-2	2	305.704	2	986.887	1
145	16	max	.001	1	.301	3	.233	1	8.687e-3	3	NC	5	NC	3
146		min	-.149	4	-.771	2	.019	12	-9.455e-3	2	316.008	2	1165.418	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	.001	1	.229	3	.153	1	7.604e-3	3	NC	5	NC	3
148			min	-.149	4	-.594	2	.014	12	-8.48e-3	2	398.401	2	1776.773	1
149		18	max	.001	1	.098	3	.079	4	6.522e-3	3	NC	5	NC	2
150			min	-.149	4	-.293	2	.006	10	-7.505e-3	2	717.661	2	3433.544	4
151		19	max	.002	1	.084	2	.007	3	5.439e-3	3	NC	1	NC	1
152			min	-.149	4	-.067	3	-.004	2	-6.53e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.008	2	.011	1	1.725e-3	5	NC	1	NC	2
154			min	-.009	3	-.014	3	-.8	4	-3.221e-4	1	9072.236	2	96.204	4
155		2	max	.007	2	.007	2	.01	1	1.836e-3	5	NC	1	NC	2
156			min	-.009	3	-.014	3	-.736	4	-3.047e-4	1	NC	1	104.614	4
157		3	max	.006	2	.006	2	.009	1	1.946e-3	5	NC	1	NC	2
158			min	-.008	3	-.013	3	-.672	4	-2.873e-4	1	NC	1	114.576	4
159		4	max	.006	2	.005	2	.008	1	2.057e-3	5	NC	1	NC	2
160			min	-.008	3	-.013	3	-.609	4	-2.699e-4	1	NC	1	126.489	4
161		5	max	.005	2	.003	2	.007	1	2.168e-3	5	NC	1	NC	1
162			min	-.007	3	-.013	3	-.547	4	-2.525e-4	1	NC	1	140.893	4
163		6	max	.005	2	.002	2	.006	1	2.279e-3	5	NC	1	NC	1
164			min	-.007	3	-.012	3	-.486	4	-2.352e-4	1	NC	1	158.533	4
165		7	max	.005	2	.001	2	.006	1	2.39e-3	5	NC	1	NC	1
166			min	-.006	3	-.011	3	-.427	4	-2.178e-4	1	NC	1	180.46	4
167		8	max	.004	2	0	2	.005	1	2.501e-3	5	NC	1	NC	1
168			min	-.006	3	-.011	3	-.37	4	-2.004e-4	1	NC	1	208.198	4
169		9	max	.004	2	0	2	.004	1	2.611e-3	5	NC	1	NC	1
170			min	-.005	3	-.01	3	-.316	4	-1.83e-4	1	NC	1	244.027	4
171		10	max	.003	2	-.001	15	.003	1	2.722e-3	5	NC	1	NC	1
172			min	-.005	3	-.01	3	-.264	4	-1.656e-4	1	NC	1	291.479	4
173		11	max	.003	2	-.001	15	.003	1	2.833e-3	5	NC	1	NC	1
174			min	-.004	3	-.009	3	-.216	4	-1.482e-4	1	NC	1	356.276	4
175		12	max	.003	2	-.001	15	.002	1	2.95e-3	4	NC	1	NC	1
176			min	-.004	3	-.008	3	-.172	4	-1.308e-4	1	NC	1	448.199	4
177		13	max	.002	2	-.001	15	.002	1	3.067e-3	4	NC	1	NC	1
178			min	-.003	3	-.007	3	-.132	4	-1.134e-4	1	NC	1	585.141	4
179		14	max	.002	2	-.001	15	.001	1	3.185e-3	4	NC	1	NC	1
180			min	-.003	3	-.006	3	-.096	4	-9.6e-5	1	NC	1	802.818	4
181		15	max	.002	2	0	15	0	1	3.302e-3	4	NC	1	NC	1
182			min	-.002	3	-.005	3	-.065	4	-7.861e-5	1	NC	1	1181.524	4
183		16	max	.001	2	0	15	0	1	3.419e-3	4	NC	1	NC	1
184			min	-.002	3	-.004	3	-.04	4	-6.122e-5	1	NC	1	1935.504	4
185		17	max	0	2	0	15	0	1	3.537e-3	4	NC	1	NC	1
186			min	-.001	3	-.003	6	-.02	4	-4.382e-5	1	NC	1	3816.995	4
187		18	max	0	2	0	15	0	1	3.654e-3	4	NC	1	NC	1
188			min	0	3	-.001	6	-.007	4	-2.643e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	3.771e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-9.034e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	1.378e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-9.745e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.018	4	2.924e-5	1	NC	1	NC	1
194			min	0	2	-.002	6	0	1	-2.274e-4	5	NC	1	9948.379	14
195		3	max	0	3	0	15	.034	4	5.288e-4	4	NC	1	NC	1
196			min	0	2	-.004	6	0	1	3.507e-6	12	NC	1	5207.275	14
197		4	max	.001	3	-.001	15	.048	4	1.28e-3	4	NC	1	NC	1
198			min	-.001	2	-.006	6	0	3	5.204e-6	12	NC	1	3629.875	14
199		5	max	.002	3	-.002	15	.062	4	2.032e-3	4	NC	1	NC	1
200			min	-.001	2	-.008	6	0	12	6.901e-6	12	NC	1	2842.017	14
201		6	max	.002	3	-.002	15	.074	4	2.784e-3	4	NC	1	NC	1
202			min	-.002	2	-.01	6	0	12	8.598e-6	12	9351.164	6	2368.579	14
203		7	max	.002	3	-.002	15	.085	4	3.535e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.011	6	0	12	1.03e-5	12	8080.224	6	2050.936	14
205		8	max	.003	3	-.003	15	.096	4	4.287e-3	4	NC	2	NC	1
206			min	-.002	2	-.012	6	0	12	1.199e-5	12	7297.823	6	1820.901	14
207		9	max	.003	3	-.003	15	.106	4	5.039e-3	4	NC	5	NC	1
208			min	-.003	2	-.013	6	0	12	1.369e-5	12	6840.67	6	1644.221	14
209		10	max	.004	3	-.003	15	.116	4	5.79e-3	4	NC	5	NC	1
210			min	-.003	2	-.014	6	0	12	1.539e-5	12	6630.074	6	1501.771	14
211		11	max	.004	3	-.003	15	.126	4	6.542e-3	4	NC	5	NC	1
212			min	-.003	2	-.014	6	0	12	1.708e-5	12	6635.269	6	1382.045	14
213		12	max	.005	3	-.003	15	.136	4	7.294e-3	4	NC	3	NC	1
214			min	-.004	2	-.013	6	0	12	1.878e-5	12	6861.612	6	1277.763	14
215		13	max	.005	3	-.003	15	.146	4	8.045e-3	4	NC	2	NC	1
216			min	-.004	2	-.012	6	0	12	2.048e-5	12	7354.173	6	1184.182	14
217		14	max	.005	3	-.002	15	.157	4	8.797e-3	4	NC	1	NC	1
218			min	-.004	2	-.011	6	0	12	2.217e-5	12	8220.592	6	1098.189	14
219		15	max	.006	3	-.002	15	.169	4	9.549e-3	4	NC	1	NC	1
220			min	-.005	2	-.01	6	0	12	2.387e-5	12	9697.555	6	1017.78	14
221		16	max	.006	3	-.001	15	.182	4	1.03e-2	4	NC	1	NC	1
222			min	-.005	2	-.008	6	0	12	2.557e-5	12	NC	1	941.725	14
223		17	max	.007	3	0	15	.197	4	1.105e-2	4	NC	1	NC	1
224			min	-.005	2	-.006	1	0	12	2.727e-5	12	NC	1	869.347	14
225		18	max	.007	3	0	15	.213	4	1.18e-2	4	NC	1	NC	1
226			min	-.006	2	-.004	3	0	12	2.896e-5	12	NC	1	800.348	14
227		19	max	.007	3	0	5	.231	4	1.256e-2	4	NC	1	NC	2
228			min	-.006	2	-.002	3	0	12	3.066e-5	12	NC	1	734.676	14
229	M4	1	max	.002	1	.006	2	0	12	3.177e-4	4	NC	1	NC	3
230			min	0	5	-.008	3	-.231	4	1.043e-5	12	NC	1	107.154	4
231		2	max	.002	1	.006	2	0	12	3.177e-4	4	NC	1	NC	3
232			min	0	5	-.007	3	-.213	4	1.043e-5	12	NC	1	116.358	4
233		3	max	.002	1	.005	2	0	12	3.177e-4	4	NC	1	NC	3
234			min	0	5	-.007	3	-.195	4	1.043e-5	12	NC	1	127.321	4
235		4	max	.002	1	.005	2	0	12	3.177e-4	4	NC	1	NC	3
236			min	0	5	-.006	3	-.177	4	1.043e-5	12	NC	1	140.499	4
237		5	max	.002	1	.005	2	0	12	3.177e-4	4	NC	1	NC	3
238			min	0	5	-.006	3	-.158	4	1.043e-5	12	NC	1	156.511	4
239		6	max	.002	1	.004	2	0	12	3.177e-4	4	NC	1	NC	3
240			min	0	5	-.006	3	-.141	4	1.043e-5	12	NC	1	176.214	4
241		7	max	.002	1	.004	2	0	12	3.177e-4	4	NC	1	NC	2
242			min	0	5	-.005	3	-.124	4	1.043e-5	12	NC	1	200.828	4
243		8	max	.001	1	.004	2	0	12	3.177e-4	4	NC	1	NC	2
244			min	0	5	-.005	3	-.107	4	1.043e-5	12	NC	1	232.127	4
245		9	max	.001	1	.003	2	0	12	3.177e-4	4	NC	1	NC	2
246			min	0	5	-.004	3	-.091	4	1.043e-5	12	NC	1	272.787	4
247		10	max	.001	1	.003	2	0	12	3.177e-4	4	NC	1	NC	2
248			min	0	5	-.004	3	-.076	4	1.043e-5	12	NC	1	326.987	4
249		11	max	.001	1	.003	2	0	12	3.177e-4	4	NC	1	NC	2
250			min	0	5	-.003	3	-.062	4	1.043e-5	12	NC	1	401.561	4
251		12	max	0	1	.002	2	0	12	3.177e-4	4	NC	1	NC	1
252			min	0	5	-.003	3	-.049	4	1.043e-5	12	NC	1	508.33	4
253		13	max	0	1	.002	2	0	12	3.177e-4	4	NC	1	NC	1
254			min	0	5	-.003	3	-.037	4	1.043e-5	12	NC	1	669.227	4
255		14	max	0	1	.002	2	0	12	3.177e-4	4	NC	1	NC	1
256			min	0	5	-.002	3	-.027	4	1.043e-5	12	NC	1	928.86	4
257		15	max	0	1	.001	2	0	12	3.177e-4	4	NC	1	NC	1
258			min	0	5	-.002	3	-.018	4	1.043e-5	12	NC	1	1390.009	4
259		16	max	0	1	0	2	0	12	3.177e-4	4	NC	1	NC	1
260			min	0	5	-.001	3	-.011	4	1.043e-5	12	NC	1	2336.647	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261	17	max	0	1	0	2	0	12	3.177e-4	4	NC	1	NC	1
262		min	0	5	0	3	-.005	4	1.043e-5	12	NC	1	4820.869	4
263	18	max	0	1	0	2	0	12	3.177e-4	4	NC	1	NC	1
264		min	0	5	0	3	-.002	4	1.043e-5	12	NC	1	NC	1
265	19	max	0	1	0	1	0	1	3.177e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	1.043e-5	12	NC	1	NC	1
267	M6	1	max	.023	2	.032	2	0	1.853e-3	4	NC	3	NC	1
268		min	-.03	3	-.045	3	-.808	4	0	1	2373.989	2	95.256	4
269	2	max	.021	2	.03	2	0	1	1.961e-3	4	NC	3	NC	1
270		min	-.028	3	-.043	3	-.743	4	0	1	2608.832	2	103.585	4
271	3	max	.02	2	.027	2	0	1	2.069e-3	4	NC	3	NC	1
272		min	-.026	3	-.04	3	-.679	4	0	1	2892.521	2	113.45	4
273	4	max	.019	2	.024	2	0	1	2.177e-3	4	NC	3	NC	1
274		min	-.025	3	-.038	3	-.615	4	0	1	3238.752	2	125.248	4
275	5	max	.018	2	.021	2	0	1	2.285e-3	4	NC	3	NC	1
276		min	-.023	3	-.035	3	-.552	4	0	1	3666.545	2	139.514	4
277	6	max	.016	2	.018	2	0	1	2.393e-3	4	NC	3	NC	1
278		min	-.021	3	-.033	3	-.491	4	0	1	4202.933	2	156.984	4
279	7	max	.015	2	.016	2	0	1	2.502e-3	4	NC	1	NC	1
280		min	-.02	3	-.03	3	-.431	4	0	1	4887.368	2	178.701	4
281	8	max	.014	2	.013	2	0	1	2.61e-3	4	NC	1	NC	1
282		min	-.018	3	-.028	3	-.373	4	0	1	5779.269	2	206.175	4
283	9	max	.013	2	.011	2	0	1	2.718e-3	4	NC	1	NC	1
284		min	-.017	3	-.025	3	-.319	4	0	1	6971.529	2	241.663	4
285	10	max	.011	2	.009	2	0	1	2.826e-3	4	NC	1	NC	1
286		min	-.015	3	-.023	3	-.267	4	0	1	8616.075	2	288.665	4
287	11	max	.01	2	.007	2	0	1	2.934e-3	4	NC	1	NC	1
288		min	-.013	3	-.02	3	-.218	4	0	1	NC	1	352.85	4
289	12	max	.009	2	.005	2	0	1	3.042e-3	4	NC	1	NC	1
290		min	-.012	3	-.018	3	-.173	4	0	1	NC	1	443.91	4
291	13	max	.008	2	.004	2	0	1	3.15e-3	4	NC	1	NC	1
292		min	-.01	3	-.015	3	-.133	4	0	1	NC	1	579.573	4
293	14	max	.006	2	.003	2	0	1	3.258e-3	4	NC	1	NC	1
294		min	-.008	3	-.012	3	-.097	4	0	1	NC	1	795.232	4
295	15	max	.005	2	.001	2	0	1	3.366e-3	4	NC	1	NC	1
296		min	-.007	3	-.01	3	-.066	4	0	1	NC	1	1170.463	4
297	16	max	.004	2	0	2	0	1	3.474e-3	4	NC	1	NC	1
298		min	-.005	3	-.007	3	-.04	4	0	1	NC	1	1917.622	4
299	17	max	.003	2	0	2	0	1	3.582e-3	4	NC	1	NC	1
300		min	-.003	3	-.005	3	-.02	4	0	1	NC	1	3782.472	4
301	18	max	.001	2	0	2	0	1	3.69e-3	4	NC	1	NC	1
302		min	-.002	3	-.002	3	-.007	4	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	3.799e-3	4	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	0	1	0	1	NC	1	NC	1
306		min	0	1	0	1	0	1	-9.813e-4	4	NC	1	NC	1
307	2	max	.001	3	0	2	.018	4	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	-2.511e-4	4	NC	1	NC	1
309	3	max	.003	3	0	15	.034	4	4.791e-4	4	NC	1	NC	1
310		min	-.003	2	-.005	3	0	1	0	1	NC	1	9703.545	4
311	4	max	.004	3	-.001	15	.049	4	1.209e-3	4	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	7752.255	4
313	5	max	.005	3	-.002	15	.062	4	1.939e-3	4	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	7132.827	4
315	6	max	.007	3	-.002	15	.074	4	2.67e-3	4	NC	1	NC	1
316		min	-.006	2	-.012	3	0	1	0	1	9067.551	3	7218.178	4
317	7	max	.008	3	-.003	15	.086	4	3.4e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.008	2	-.014	3	0	1	0	1	8093.744	3	7935.984	4
319	8	max	.009	3	-.003	15	.096	4	4.13e-3	4	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7359.591	4	9551.034	4
321	9	max	.01	3	-.003	15	.106	4	4.86e-3	4	NC	1	NC	1
322		min	-.01	2	-.016	3	0	1	0	1	6895.085	4	NC	1
323	10	max	.012	3	-.003	15	.116	4	5.59e-3	4	NC	1	NC	1
324		min	-.012	2	-.016	3	0	1	0	1	6679.959	4	NC	1
325	11	max	.013	3	-.003	15	.125	4	6.32e-3	4	NC	1	NC	1
326		min	-.013	2	-.016	3	0	1	0	1	6682.77	4	NC	1
327	12	max	.014	3	-.003	15	.134	4	7.051e-3	4	NC	1	NC	1
328		min	-.014	2	-.016	3	0	1	0	1	6908.61	4	NC	1
329	13	max	.016	3	-.003	15	.144	4	7.781e-3	4	NC	1	NC	1
330		min	-.015	2	-.016	3	0	1	0	1	7402.623	4	NC	1
331	14	max	.017	3	-.003	15	.154	4	8.511e-3	4	NC	1	NC	1
332		min	-.017	2	-.015	3	0	1	0	1	8272.957	4	NC	1
333	15	max	.018	3	-.002	15	.165	4	9.241e-3	4	NC	1	NC	1
334		min	-.018	2	-.014	3	0	1	0	1	9757.596	4	NC	1
335	16	max	.02	3	-.002	15	.177	4	9.971e-3	4	NC	1	NC	1
336		min	-.019	2	-.012	3	0	1	0	1	NC	1	NC	1
337	17	max	.021	3	-.001	10	.191	4	1.07e-2	4	NC	1	NC	1
338		min	-.02	2	-.011	3	0	1	0	1	NC	1	NC	1
339	18	max	.022	3	0	2	.206	4	1.143e-2	4	NC	1	NC	1
340		min	-.022	2	-.009	3	0	1	0	1	NC	1	NC	1
341	19	max	.023	3	.002	2	.223	4	1.216e-2	4	NC	1	NC	1
342		min	-.023	2	-.007	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	1	.023	2	0	1.322e-4	5	NC	1	NC	1
344		min	0	15	-.025	3	-.223	4	0	1	NC	1	111.229	4
345	2	max	.005	1	.021	2	0	1	1.322e-4	5	NC	1	NC	1
346		min	0	15	-.023	3	-.205	4	0	1	NC	1	120.796	4
347	3	max	.005	1	.02	2	0	1	1.322e-4	5	NC	1	NC	1
348		min	0	15	-.022	3	-.188	4	0	1	NC	1	132.192	4
349	4	max	.005	1	.019	2	0	1	1.322e-4	5	NC	1	NC	1
350		min	0	15	-.02	3	-.17	4	0	1	NC	1	145.888	4
351	5	max	.004	1	.018	2	0	1	1.322e-4	5	NC	1	NC	1
352		min	0	15	-.019	3	-.153	4	0	1	NC	1	162.53	4
353	6	max	.004	1	.016	2	0	1	1.322e-4	5	NC	1	NC	1
354		min	0	15	-.018	3	-.136	4	0	1	NC	1	183.007	4
355	7	max	.004	1	.015	2	0	1	1.322e-4	5	NC	1	NC	1
356		min	0	15	-.016	3	-.119	4	0	1	NC	1	208.587	4
357	8	max	.004	1	.014	2	0	1	1.322e-4	5	NC	1	NC	1
358		min	0	15	-.015	3	-.103	4	0	1	NC	1	241.115	4
359	9	max	.003	1	.013	2	0	1	1.322e-4	5	NC	1	NC	1
360		min	0	15	-.014	3	-.088	4	0	1	NC	1	283.372	4
361	10	max	.003	1	.011	2	0	1	1.322e-4	5	NC	1	NC	1
362		min	0	15	-.012	3	-.073	4	0	1	NC	1	339.699	4
363	11	max	.003	1	.01	2	0	1	1.322e-4	5	NC	1	NC	1
364		min	0	15	-.011	3	-.059	4	0	1	NC	1	417.202	4
365	12	max	.002	1	.009	2	0	1	1.322e-4	5	NC	1	NC	1
366		min	0	15	-.01	3	-.047	4	0	1	NC	1	528.166	4
367	13	max	.002	1	.008	2	0	1	1.322e-4	5	NC	1	NC	1
368		min	0	15	-.008	3	-.036	4	0	1	NC	1	695.388	4
369	14	max	.002	1	.006	2	0	1	1.322e-4	5	NC	1	NC	1
370		min	0	15	-.007	3	-.026	4	0	1	NC	1	965.233	4
371	15	max	.001	1	.005	2	0	1	1.322e-4	5	NC	1	NC	1
372		min	0	15	-.005	3	-.017	4	0	1	NC	1	1444.536	4
373	16	max	0	1	.004	2	0	1	1.322e-4	5	NC	1	NC	1
374		min	0	15	-.004	3	-.01	4	0	1	NC	1	2428.478	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.003	2	0	1	1.322e-4	5	NC	1	NC	1
376			min	0	15	-.003	3	-.005	4	0	1	NC	1	5010.734	4
377		18	max	0	1	.001	2	0	1	1.322e-4	5	NC	1	NC	1
378			min	0	15	-.001	3	-.002	4	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	1.322e-4	5	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.008	2	0	12	1.878e-3	4	NC	1	NC	2
382			min	-.009	3	-.014	3	-.808	4	2.151e-5	12	9072.236	2	95.329	4
383		2	max	.007	2	.007	2	0	12	1.984e-3	4	NC	1	NC	2
384			min	-.009	3	-.014	3	-.743	4	2.035e-5	12	NC	1	103.665	4
385		3	max	.006	2	.006	2	0	12	2.089e-3	4	NC	1	NC	2
386			min	-.008	3	-.013	3	-.678	4	1.919e-5	12	NC	1	113.539	4
387		4	max	.006	2	.005	2	0	12	2.195e-3	4	NC	1	NC	2
388			min	-.008	3	-.013	3	-.614	4	1.803e-5	12	NC	1	125.348	4
389		5	max	.005	2	.003	2	0	12	2.3e-3	4	NC	1	NC	1
390			min	-.007	3	-.013	3	-.552	4	1.688e-5	12	NC	1	139.627	4
391		6	max	.005	2	.002	2	0	12	2.406e-3	4	NC	1	NC	1
392			min	-.007	3	-.012	3	-.49	4	1.572e-5	12	NC	1	157.114	4
393		7	max	.005	2	.001	2	0	12	2.511e-3	4	NC	1	NC	1
394			min	-.006	3	-.011	3	-.431	4	1.456e-5	12	NC	1	178.852	4
395		8	max	.004	2	0	2	0	12	2.617e-3	4	NC	1	NC	1
396			min	-.006	3	-.011	3	-.373	4	1.34e-5	12	NC	1	206.354	4
397		9	max	.004	2	0	2	0	12	2.722e-3	4	NC	1	NC	1
398			min	-.005	3	-.01	3	-.318	4	1.224e-5	12	NC	1	241.88	4
399		10	max	.003	2	-.001	2	0	12	2.828e-3	4	NC	1	NC	1
400			min	-.005	3	-.01	3	-.267	4	1.108e-5	12	NC	1	288.934	4
401		11	max	.003	2	-.002	2	0	12	2.933e-3	4	NC	1	NC	1
402			min	-.004	3	-.009	3	-.218	4	9.927e-6	12	NC	1	353.194	4
403		12	max	.003	2	-.002	15	0	12	3.039e-3	4	NC	1	NC	1
404			min	-.004	3	-.008	3	-.173	4	8.768e-6	12	NC	1	444.367	4
405		13	max	.002	2	-.002	15	0	12	3.144e-3	4	NC	1	NC	1
406			min	-.003	3	-.007	3	-.133	4	7.61e-6	12	NC	1	580.21	4
407		14	max	.002	2	-.001	15	0	12	3.25e-3	4	NC	1	NC	1
408			min	-.003	3	-.006	3	-.097	4	6.452e-6	12	NC	1	796.183	4
409		15	max	.002	2	-.001	15	0	12	3.355e-3	4	NC	1	NC	1
410			min	-.002	3	-.005	4	-.066	4	5.293e-6	12	NC	1	1172.022	4
411		16	max	.001	2	-.001	15	0	12	3.461e-3	4	NC	1	NC	1
412			min	-.002	3	-.004	4	-.04	4	4.135e-6	12	NC	1	1920.576	4
413		17	max	0	2	0	15	0	12	3.566e-3	4	NC	1	NC	1
414			min	-.001	3	-.003	4	-.02	4	2.977e-6	12	NC	1	3789.637	4
415		18	max	0	2	0	15	0	12	3.672e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.007	4	1.818e-6	12	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.777e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	6.602e-7	12	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-1.133e-7	12	NC	1	NC	1
420			min	0	1	0	1	0	1	-9.753e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.018	4	-1.81e-6	12	NC	1	NC	1
422			min	0	2	-.002	4	0	12	-2.426e-4	4	NC	1	NC	1
423		3	max	0	3	-.001	15	.034	4	4.963e-4	5	NC	1	NC	1
424			min	0	2	-.004	4	0	12	-5.711e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	.048	4	1.225e-3	5	NC	1	NC	1
426			min	-.001	2	-.006	4	0	1	-8.497e-5	1	NC	1	8135.76	4
427		5	max	.002	3	-.002	15	.062	4	1.956e-3	4	NC	1	NC	1
428			min	-.001	2	-.008	4	0	1	-1.128e-4	1	NC	1	7559.696	4
429		6	max	.002	3	-.003	15	.074	4	2.688e-3	4	NC	1	NC	1
430			min	-.002	2	-.01	4	0	1	-1.407e-4	1	9131.331	4	7755.258	4
431		7	max	.002	3	-.003	15	.085	4	3.421e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432		min	-.002	2	-.012	4	0	1	-1.686e-4	1	7903.969	4	8700.84	4
433	8	max	.003	3	-.003	15	.096	4	4.154e-3	4	NC	2	NC	1
434		min	-.002	2	-.013	4	0	1	-1.964e-4	1	7148.975	4	NC	1
435	9	max	.003	3	-.003	15	.105	4	4.886e-3	4	NC	5	NC	1
436		min	-.003	2	-.014	4	-.001	1	-2.243e-4	1	6709.316	4	NC	1
437	10	max	.004	3	-.004	15	.115	4	5.619e-3	4	NC	5	NC	1
438		min	-.003	2	-.015	4	-.001	1	-2.522e-4	1	6509.478	4	NC	1
439	11	max	.004	3	-.004	15	.124	4	6.352e-3	4	NC	5	NC	1
440		min	-.003	2	-.015	4	-.002	1	-2.8e-4	1	6520.293	4	NC	1
441	12	max	.005	3	-.004	15	.134	4	7.084e-3	4	NC	3	NC	1
442		min	-.004	2	-.014	4	-.003	1	-3.079e-4	1	6747.736	4	NC	1
443	13	max	.005	3	-.003	15	.144	4	7.817e-3	4	NC	2	NC	1
444		min	-.004	2	-.013	4	-.003	1	-3.358e-4	1	7236.674	4	NC	1
445	14	max	.005	3	-.003	15	.154	4	8.55e-3	4	NC	1	NC	1
446		min	-.004	2	-.012	4	-.004	1	-3.636e-4	1	8093.506	4	NC	1
447	15	max	.006	3	-.003	15	.165	4	9.283e-3	4	NC	1	NC	1
448		min	-.005	2	-.011	4	-.005	1	-3.915e-4	1	9551.75	4	9185.048	5
449	16	max	.006	3	-.002	15	.178	4	1.002e-2	4	NC	1	NC	1
450		min	-.005	2	-.009	4	-.006	1	-4.194e-4	1	NC	1	9215.828	5
451	17	max	.007	3	-.002	15	.192	4	1.075e-2	4	NC	1	NC	1
452		min	-.005	2	-.006	4	-.007	1	-4.472e-4	1	NC	1	NC	1
453	18	max	.007	3	-.001	15	.207	4	1.148e-2	4	NC	1	NC	1
454		min	-.006	2	-.004	4	-.009	1	-4.751e-4	1	NC	1	NC	1
455	19	max	.007	3	0	10	.225	4	1.221e-2	4	NC	1	NC	2
456		min	-.006	2	-.002	3	-.01	1	-5.03e-4	1	NC	1	8756.742	1
457	M12	1	max	.002	1	.006	2	.01	2.447e-4	5	NC	1	NC	3
458		min	0	12	-.008	3	-.225	4	-1.665e-4	1	NC	1	110.479	4
459	2	max	.002	1	.006	2	.009	1	2.447e-4	5	NC	1	NC	3
460		min	0	12	-.007	3	-.207	4	-1.665e-4	1	NC	1	119.974	4
461	3	max	.002	1	.005	2	.009	1	2.447e-4	5	NC	1	NC	3
462		min	0	12	-.007	3	-.189	4	-1.665e-4	1	NC	1	131.284	4
463	4	max	.002	1	.005	2	.008	1	2.447e-4	5	NC	1	NC	3
464		min	0	12	-.006	3	-.171	4	-1.665e-4	1	NC	1	144.878	4
465	5	max	.002	1	.005	2	.007	1	2.447e-4	5	NC	1	NC	3
466		min	0	12	-.006	3	-.154	4	-1.665e-4	1	NC	1	161.395	4
467	6	max	.002	1	.004	2	.006	1	2.447e-4	5	NC	1	NC	3
468		min	0	12	-.006	3	-.136	4	-1.665e-4	1	NC	1	181.72	4
469	7	max	.002	1	.004	2	.006	1	2.447e-4	5	NC	1	NC	2
470		min	0	12	-.005	3	-.12	4	-1.665e-4	1	NC	1	207.109	4
471	8	max	.001	1	.004	2	.005	1	2.447e-4	5	NC	1	NC	2
472		min	0	12	-.005	3	-.104	4	-1.665e-4	1	NC	1	239.396	4
473	9	max	.001	1	.003	2	.004	1	2.447e-4	5	NC	1	NC	2
474		min	0	12	-.004	3	-.088	4	-1.665e-4	1	NC	1	281.338	4
475	10	max	.001	1	.003	2	.003	1	2.447e-4	5	NC	1	NC	2
476		min	0	12	-.004	3	-.074	4	-1.665e-4	1	NC	1	337.247	4
477	11	max	.001	1	.003	2	.003	1	2.447e-4	5	NC	1	NC	2
478		min	0	12	-.003	3	-.06	4	-1.665e-4	1	NC	1	414.173	4
479	12	max	0	1	.002	2	.002	1	2.447e-4	5	NC	1	NC	1
480		min	0	12	-.003	3	-.047	4	-1.665e-4	1	NC	1	524.31	4
481	13	max	0	1	.002	2	.002	1	2.447e-4	5	NC	1	NC	1
482		min	0	12	-.003	3	-.036	4	-1.665e-4	1	NC	1	690.284	4
483	14	max	0	1	.002	2	.001	1	2.447e-4	5	NC	1	NC	1
484		min	0	12	-.002	3	-.026	4	-1.665e-4	1	NC	1	958.111	4
485	15	max	0	1	.001	2	0	1	2.447e-4	5	NC	1	NC	1
486		min	0	12	-.002	3	-.017	4	-1.665e-4	1	NC	1	1433.82	4
487	16	max	0	1	0	2	0	1	2.447e-4	5	NC	1	NC	1
488		min	0	12	-.001	3	-.01	4	-1.665e-4	1	NC	1	2410.364	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	2.447e-4	5	NC	1	NC	1
490			min	0	12	0	3	-.005	4	-1.665e-4	1	NC	1	4973.121	4
491		18	max	0	1	0	2	0	1	2.447e-4	5	NC	1	NC	1
492			min	0	12	0	3	-.002	4	-1.665e-4	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	2.447e-4	5	NC	1	NC	1
494			min	0	1	0	1	0	1	-1.665e-4	1	NC	1	NC	1
495	M1	1	max	.009	3	.095	2	.86	4	1.668e-2	2	NC	1	NC	1
496			min	-.005	2	-.014	3	0	12	-2.978e-2	3	NC	1	NC	1
497		2	max	.009	3	.044	2	.83	4	8.876e-3	4	NC	4	NC	1
498			min	-.005	2	-.003	3	-.007	1	-1.474e-2	3	2255.144	2	9651.949	5
499		3	max	.009	3	.015	3	.8	4	1.43e-2	4	NC	5	NC	2
500			min	-.005	2	-.011	2	-.011	1	-2.276e-4	1	1086.094	2	5270.066	5
501		4	max	.009	3	.045	3	.769	4	1.249e-2	4	NC	5	NC	1
502			min	-.005	2	-.073	2	-.01	1	-5.271e-3	3	684.917	2	3803.319	5
503		5	max	.009	3	.084	3	.737	4	1.068e-2	4	NC	5	NC	1
504			min	-.005	2	-.138	2	-.007	1	-1.039e-2	3	493.898	2	3072.809	5
505		6	max	.009	3	.125	3	.705	4	1.368e-2	2	NC	15	NC	1
506			min	-.004	2	-.201	2	-.003	1	-1.551e-2	3	388.744	2	2635.32	5
507		7	max	.008	3	.165	3	.672	4	1.825e-2	2	NC	15	NC	1
508			min	-.004	2	-.258	2	0	12	-2.063e-2	3	326.711	2	2322.261	4
509		8	max	.008	3	.199	3	.638	4	2.282e-2	2	9442.704	15	NC	1
510			min	-.004	2	-.302	2	0	12	-2.575e-2	3	290.04	2	2094.059	4
511		9	max	.008	3	.22	3	.602	4	2.635e-2	2	8819.473	15	NC	1
512			min	-.004	2	-.331	2	0	1	-2.589e-2	3	270.959	2	1953.563	4
513		10	max	.008	3	.228	3	.564	4	2.918e-2	2	8629.822	15	NC	1
514			min	-.004	2	-.34	2	0	12	-2.271e-2	3	265.387	2	1913.322	4
515		11	max	.008	3	.222	3	.523	4	3.202e-2	2	8819.121	15	NC	1
516			min	-.004	2	-.33	2	0	12	-1.952e-2	3	271.994	2	1957.858	4
517		12	max	.007	3	.203	3	.48	4	3.127e-2	2	9441.912	15	NC	1
518			min	-.004	2	-.301	2	-.001	1	-1.631e-2	3	293.219	2	2100.189	4
519		13	max	.007	3	.173	3	.432	4	2.509e-2	2	NC	15	NC	1
520			min	-.004	2	-.253	2	0	1	-1.305e-2	3	334.514	2	2455.611	4
521		14	max	.007	3	.134	3	.382	4	1.891e-2	2	NC	15	NC	1
522			min	-.004	2	-.194	2	0	12	-9.796e-3	3	405.52	2	3182.394	4
523		15	max	.007	3	.092	3	.331	4	1.274e-2	2	NC	5	NC	1
524			min	-.004	2	-.13	2	0	12	-6.538e-3	3	528.651	2	4717.175	4
525		16	max	.007	3	.047	3	.281	4	9.743e-3	4	NC	5	NC	1
526			min	-.004	2	-.065	2	0	12	-3.28e-3	3	758.755	2	8659.552	4
527		17	max	.007	3	.005	3	.233	4	1.093e-2	4	NC	5	NC	2
528			min	-.004	2	-.006	2	0	12	-2.187e-5	3	1255.057	2	9868.238	1
529		18	max	.007	3	.042	2	.189	4	1.3e-2	2	NC	4	NC	1
530			min	-.004	2	-.032	3	0	12	-5.632e-3	3	2688.138	2	NC	1
531		19	max	.007	3	.084	2	.149	4	2.605e-2	2	NC	1	NC	1
532			min	-.004	2	-.067	3	-.002	1	-1.145e-2	3	NC	1	NC	1
533	M5	1	max	.029	3	.234	2	.859	4	0	1	NC	1	NC	1
534			min	-.02	2	-.023	3	0	1	-5.563e-6	4	NC	1	NC	1
535		2	max	.029	3	.105	2	.836	4	7.363e-3	4	NC	5	NC	1
536			min	-.02	2	.001	3	0	1	0	1	899.396	2	7154.88	4
537		3	max	.029	3	.048	3	.808	4	1.45e-2	4	NC	5	NC	1
538			min	-.02	2	-.038	2	0	1	0	1	425.636	2	4195.454	4
539		4	max	.028	3	.136	3	.776	4	1.182e-2	4	9709.615	15	NC	1
540			min	-.02	2	-.206	2	0	1	0	1	262.558	2	3257.279	4
541		5	max	.027	3	.252	3	.742	4	9.129e-3	4	6801.905	15	NC	1
542			min	-.019	2	-.387	2	0	1	0	1	185.962	2	2816.662	4
543		6	max	.027	3	.379	3	.707	4	6.442e-3	4	5240.639	15	NC	1
544			min	-.019	2	-.565	2	0	1	0	1	144.411	2	2553.018	4
545		7	max	.026	3	.503	3	.672	4	3.755e-3	4	4338.228	15	NC	1





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Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.018	2	-.726	2	0	1	0	1	120.196	2	2345.512	4
547		8	max	.026	3	.605	3	.637	4	1.068e-3	4	3813.179	15	NC	1
548			min	-.018	2	-.855	2	0	1	0	1	106.023	2	2134.571	4
549		9	max	.025	3	.671	3	.603	4	0	1	3543.79	15	NC	1
550			min	-.018	2	-.936	2	0	1	-4.046e-6	5	98.718	2	1948.64	4
551		10	max	.025	3	.694	3	.564	4	0	1	3462.622	15	NC	1
552			min	-.017	2	-.964	2	0	1	-3.933e-6	5	96.588	2	1924.719	4
553		11	max	.024	3	.676	3	.523	4	0	1	3543.901	15	NC	1
554			min	-.017	2	-.936	2	0	1	-3.82e-6	5	99.107	2	1980.125	4
555		12	max	.023	3	.618	3	.481	4	7.675e-4	4	3813.446	15	NC	1
556			min	-.017	2	-.85	2	0	1	0	1	107.286	2	2060.914	4
557		13	max	.023	3	.524	3	.434	4	2.7e-3	4	4338.794	15	NC	1
558			min	-.017	2	-.713	2	0	1	0	1	123.449	2	2423.16	4
559		14	max	.022	3	.406	3	.381	4	4.632e-3	4	5241.777	15	NC	1
560			min	-.016	2	-.543	2	0	1	0	1	151.7	2	3371.724	4
561		15	max	.022	3	.275	3	.326	4	6.564e-3	4	6804.194	15	NC	1
562			min	-.016	2	-.358	2	0	1	0	1	201.74	2	6100.415	4
563		16	max	.021	3	.141	3	.273	4	8.496e-3	4	9714.46	15	NC	1
564			min	-.016	2	-.178	2	0	1	0	1	297.898	2	NC	1
565		17	max	.02	3	.016	3	.224	4	1.043e-2	4	NC	5	NC	1
566			min	-.016	2	-.02	2	0	1	0	1	511.806	2	NC	1
567		18	max	.02	3	.1	2	.183	4	5.295e-3	4	NC	5	NC	1
568			min	-.016	2	-.091	3	0	1	0	1	1131.042	2	NC	1
569		19	max	.02	3	.199	2	.15	4	0	1	NC	1	NC	1
570			min	-.016	2	-.188	3	0	1	-3.433e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.095	2	.859	4	2.978e-2	3	NC	1	NC	1
572			min	-.005	2	-.014	3	-.001	1	-1.668e-2	2	NC	1	NC	1
573		2	max	.009	3	.044	2	.835	4	1.474e-2	3	NC	4	NC	1
574			min	-.005	2	-.003	3	0	12	-8.174e-3	2	2255.144	2	7250.024	4
575		3	max	.009	3	.015	3	.807	4	1.449e-2	4	NC	5	NC	2
576			min	-.005	2	-.011	2	0	12	-9.135e-6	10	1086.094	2	4220.079	4
577		4	max	.009	3	.045	3	.776	4	1.135e-2	5	NC	5	NC	1
578			min	-.005	2	-.073	2	0	12	-4.535e-3	2	684.917	2	3251.12	4
579		5	max	.009	3	.084	3	.742	4	1.039e-2	3	NC	5	NC	1
580			min	-.005	2	-.138	2	0	12	-9.106e-3	2	493.898	2	2792.448	4
581		6	max	.009	3	.125	3	.707	4	1.551e-2	3	NC	15	NC	1
582			min	-.004	2	-.201	2	0	12	-1.368e-2	2	388.744	2	2520.405	4
583		7	max	.008	3	.165	3	.672	4	2.063e-2	3	NC	15	NC	1
584			min	-.004	2	-.258	2	0	1	-1.825e-2	2	326.711	2	2315.538	4
585		8	max	.008	3	.199	3	.637	4	2.575e-2	3	9419.047	15	NC	1
586			min	-.004	2	-.302	2	-.001	1	-2.282e-2	2	290.04	2	2119.196	4
587		9	max	.008	3	.22	3	.602	4	2.589e-2	3	8797.688	15	NC	1
588			min	-.004	2	-.331	2	0	12	-2.635e-2	2	270.959	2	1946.928	4
589		10	max	.008	3	.228	3	.564	4	2.271e-2	3	8608.591	15	NC	1
590			min	-.004	2	-.34	2	0	1	-2.918e-2	2	265.387	2	1914.832	4
591		11	max	.008	3	.222	3	.523	4	1.952e-2	3	8797.367	15	NC	1
592			min	-.004	2	-.33	2	0	1	-3.202e-2	2	271.994	2	1967.115	4
593		12	max	.007	3	.203	3	.481	4	1.631e-2	3	9418.445	15	NC	1
594			min	-.004	2	-.301	2	0	12	-3.127e-2	2	293.219	2	2076.161	4
595		13	max	.007	3	.173	3	.433	4	1.305e-2	3	NC	15	NC	1
596			min	-.004	2	-.253	2	0	12	-2.509e-2	2	334.514	2	2459.295	4
597		14	max	.007	3	.134	3	.38	4	9.796e-3	3	NC	15	NC	1
598			min	-.004	2	-.194	2	-.003	1	-1.891e-2	2	405.52	2	3341.284	5
599		15	max	.007	3	.092	3	.327	4	6.538e-3	3	NC	5	NC	1
600			min	-.004	2	-.13	2	-.007	1	-1.274e-2	2	528.651	2	5459.205	5
601		16	max	.007	3	.047	3	.274	4	8.35e-3	5	NC	5	NC	1
602			min	-.004	2	-.065	2	-.01	1	-6.561e-3	2	758.755	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.007	3	.005	3	.226	4	1.051e-2	4	NC	5	NC	2
604		min	-.004	2	-.006	2	-.01	1	-6.76e-4	1	1255.057	2	9868.238	1
605	18	max	.007	3	.042	2	.184	4	5.632e-3	3	NC	4	NC	1
606		min	-.004	2	-.032	3	-.007	1	-1.3e-2	2	2688.138	2	NC	1
607	19	max	.007	3	.084	2	.15	4	1.145e-2	3	NC	1	NC	1
608		min	-.004	2	-.067	3	0	12	-2.605e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Software  
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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

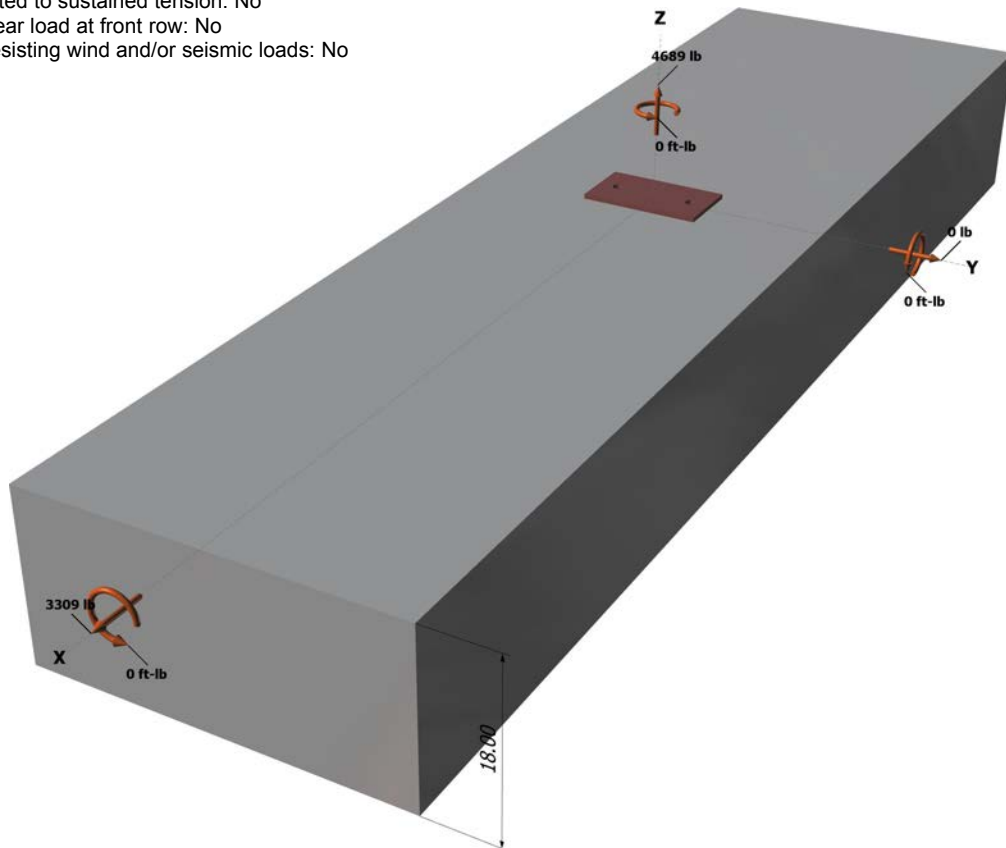
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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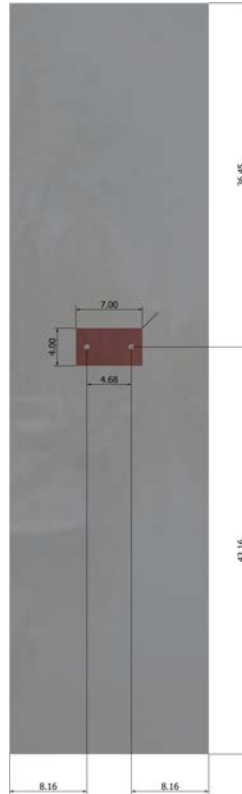




Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

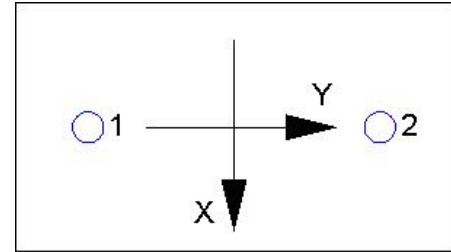
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.