

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	25° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	129 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.876 k-ft
M_z =	0.379 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	100%

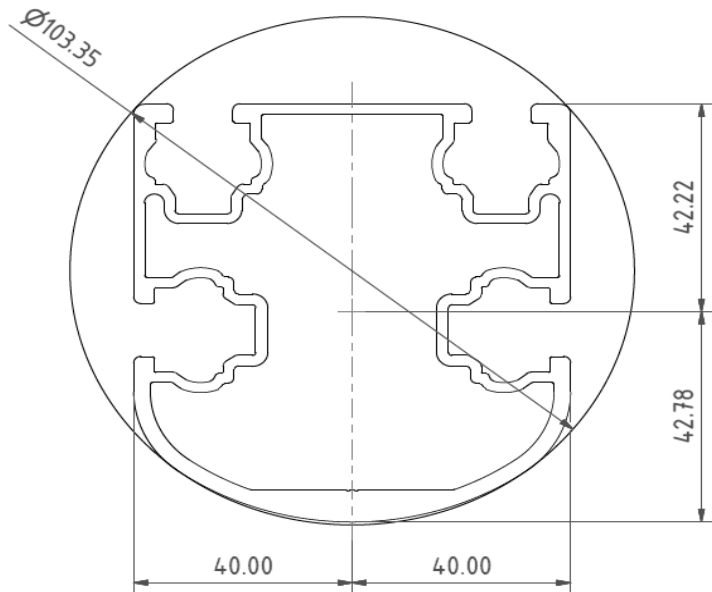


DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	88.90 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.35 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-2.777 k-ft
M_z =	0.000 k-ft
P_n =	-0.225 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	81%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	3.028 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	11%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.009 k-ft
M_z =	0.000 k-ft
P_n =	1.693 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	24%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	63.42 in
$\Phi F_{ty \text{ AXIAL}}$ =	12.77 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.008 k-ft
M_z =	0.000 k-ft
P_n =	3.184 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	12.545 k
Utilization =	26%



5. FOUNDATION DESIGN CALCULATIONS

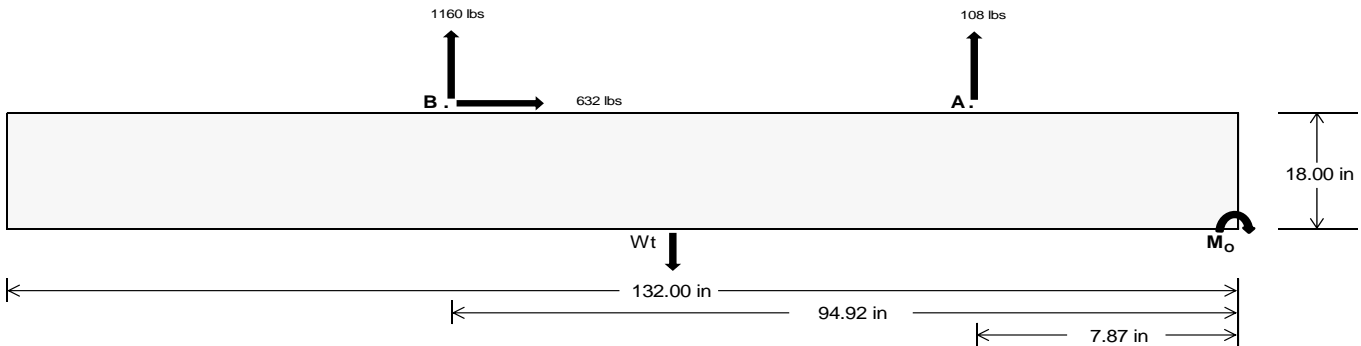
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		464.45	4838.59 k
Compressive Load =		3937.01	4451.22 k
Lateral Load =		16.08	2629.64 k
Moment (Weak Axis) =		0.03	0.01 k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 122334.7$ in-lbs
Resisting Force Required = 1853.56 lbs
S.F. = 1.67
Weight Required = 3089.26 lbs
Minimum Width = 26 in
Weight Provided = 5183.75 lbs

Sliding

Force = 631.79 lbs
Friction = 0.4
Weight Required = 1579.48 lbs
Resisting Weight = 5183.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 631.79 lbs
Cohesion = 130 psf
Area = 23.83 ft²
Resisting = 2591.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 26in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 26in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 26in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.17 \text{ ft}) =$
26 in 27 in 28 in 29 in
5184 lbs 5383 lbs 5583 lbs 5782 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	26 in	27 in	28 in	29 in	26 in	27 in	28 in	29 in	26 in	27 in	28 in	29 in	26 in	27 in	28 in	29 in
F_A	1489 lbs	1489 lbs	1489 lbs	1489 lbs	1223 lbs	1223 lbs	1223 lbs	1223 lbs	1896 lbs	1896 lbs	1896 lbs	1896 lbs	-217 lbs	-217 lbs	-217 lbs	-217 lbs
F_B	1481 lbs	1481 lbs	1481 lbs	1481 lbs	1725 lbs	1725 lbs	1725 lbs	1725 lbs	2268 lbs	2268 lbs	2268 lbs	2268 lbs	-2320 lbs	-2320 lbs	-2320 lbs	-2320 lbs
F_V	204 lbs	204 lbs	204 lbs	204 lbs	1152 lbs	1152 lbs	1152 lbs	1152 lbs	1000 lbs	1000 lbs	1000 lbs	1000 lbs	-1264 lbs	-1264 lbs	-1264 lbs	-1264 lbs
P_{total}	8154 lbs	8353 lbs	8553 lbs	8752 lbs	8133 lbs	8332 lbs	8531 lbs	8731 lbs	9348 lbs	9548 lbs	9747 lbs	9947 lbs	574 lbs	693 lbs	813 lbs	932 lbs
M	3949 lbs-ft	3949 lbs-ft	3949 lbs-ft	3949 lbs-ft	3495 lbs-ft	3495 lbs-ft	3495 lbs-ft	3495 lbs-ft	5218 lbs-ft	5218 lbs-ft	5218 lbs-ft	5218 lbs-ft	2647 lbs-ft	2647 lbs-ft	2647 lbs-ft	2647 lbs-ft
e	0.48 ft	0.47 ft	0.46 ft	0.45 ft	0.43 ft	0.42 ft	0.41 ft	0.40 ft	0.56 ft	0.55 ft	0.54 ft	0.52 ft	4.61 ft	3.82 ft	3.26 ft	2.84 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	251.7 psf	250.5 psf	249.3 psf	248.2 psf	261.2 psf	259.6 psf	258.1 psf	256.7 psf	272.8 psf	270.8 psf	268.9 psf	267.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	432.5 psf	424.6 psf	417.2 psf	410.3 psf	421.2 psf	413.7 psf	406.7 psf	400.1 psf	511.7 psf	500.8 psf	490.7 psf	481.2 psf	199.2 psf	122.1 psf	103.5 psf	96.6 psf

Maximum Bearing Pressure = 512 psf
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 26in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

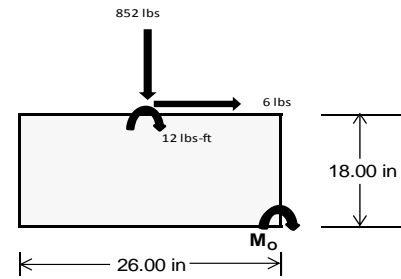
Overturning Check

$M_o = 901.1 \text{ ft-lbs}$
 Resisting Force Required = 831.83 lbs
 S.F. = 1.67
 Weight Required = 1386.38 lbs
 Minimum Width = **26 in**
 Weight Provided = 5183.75 lbs

A minimum 132in long x 26in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	26 in			26 in			26 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	259 lbs	680 lbs	259 lbs	852 lbs	2463 lbs	852 lbs	76 lbs	199 lbs	76 lbs
F_h	2 lbs	0 lbs	2 lbs	6 lbs	0 lbs	6 lbs	0 lbs	0 lbs	0 lbs
P_{total}	6677 lbs	5184 lbs	6677 lbs	6961 lbs	5184 lbs	6961 lbs	1952 lbs	5184 lbs	1952 lbs
M	6 lbs-ft	0 lbs-ft	6 lbs-ft	21 lbs-ft	0 lbs-ft	21 lbs-ft	2 lbs-ft	0 lbs-ft	2 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.36 ft	0.36 ft	0.36 ft	0.36 ft	0.36 ft	0.36 ft	0.36 ft	0.36 ft	0.36 ft
f_{min}	279.4 psf	217.5 psf	279.4 psf	289.6 psf	217.5 psf	289.6 psf	81.7 psf	217.5 psf	81.7 psf
f_{max}	280.9 psf	217.5 psf	280.9 psf	294.6 psf	217.5 psf	294.6 psf	82.1 psf	217.5 psf	82.1 psf



Maximum Bearing Pressure = 295 psf
 Allowable Bearing Pressure = 1500 psf

Use a 132in long x 26in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 132in long x 26in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.474 k
Allowable Uplift =	1.214 k
Utilization =	<u>39%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.799 k
Allowable Uplift =	4.357 k
Utilization =	<u>41%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	3.028 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>41%</u>

Rear Strut

Maximum Axial Load =	3.269 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>44%</u>

Diagonal Strut

Maximum Axial Load =	1.754 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>24%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	46.89 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	0.938 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 129 \text{ in}$$

$$J = 0.432$$

$$356.874$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 27.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 129$$

$$J = 0.432$$

$$226.951$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 28.4$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

Strong Axis:

3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 63.42 \text{ in} \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 63.42 \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.46712$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7854$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 12.7711 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 12.77 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 13.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.9	-46.9	0	0
2	M14	Y	-46.9	-46.9	0	0
3	M15	Y	-46.9	-46.9	0	0
4	M16	Y	-46.9	-46.9	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-39.013	-39.013	0	0
2	M14	y	-39.013	-39.013	0	0
3	M15	y	-60.293	-60.293	0	0
4	M16	y	-60.293	-60.293	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	88.666	88.666	0	0
2	M14	y	67.386	67.386	0	0
3	M15	y	35.466	35.466	0	0
4	M16	y	35.466	35.466	0	0

Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	126.551	1	217.378	1	-749	12	.012	1	-0.007	15	.857	3
28			min	5.228	15	-265.707	3	-29.154	1	0	3	-.178	1	-.663	1
29		15	max	126.551	1	86.044	1	14.705	1	.012	1	-.008	12	1.076	3
30			min	5.228	15	-102.057	3	.616	15	0	3	-.187	1	-.844	1
31		16	max	126.551	1	61.592	3	58.563	1	.012	1	-.005	12	1.1	3
32			min	5.228	15	-45.291	1	2.419	15	0	3	-.143	1	-.868	1
33		17	max	126.551	1	225.242	3	102.422	1	.012	1	0	12	.929	3
34			min	5.228	15	-176.625	1	4.223	15	0	3	-.047	1	-.736	1
35		18	max	126.551	1	388.891	3	146.28	1	.012	1	.101	1	.562	3
36			min	5.228	15	-307.959	1	6.027	15	0	3	.004	15	-.446	1
37		19	max	126.551	1	552.541	3	190.139	1	.012	1	.302	1	0	1
38			min	5.228	15	-439.293	1	7.83	15	0	3	.012	15	0	3
39	M14	1	max	56.238	1	460.524	1	-8.058	15	.006	3	.342	1	0	1
40			min	2.329	15	-424.931	3	-195.689	1	-.009	1	.014	15	0	3
41		2	max	56.238	1	329.19	1	-6.255	15	.006	3	.135	1	.434	3
42			min	2.329	15	-301.95	3	-151.831	1	-.009	1	.006	15	-.472	1
43		3	max	56.238	1	197.856	1	-4.451	15	.006	3	0	3	.721	3
44			min	2.329	15	-178.969	3	-107.972	1	-.009	1	-.021	1	-.786	1
45		4	max	56.238	1	66.522	1	-2.647	15	.006	3	-.004	12	.862	3
46			min	2.329	15	-55.988	3	-64.114	1	-.009	1	-.123	1	-.944	1
47		5	max	56.238	1	66.993	3	-.844	15	.006	3	-.007	12	.855	3
48			min	2.329	15	-64.813	1	-20.255	1	-.009	1	-.174	1	-.945	1
49		6	max	56.238	1	189.974	3	23.604	1	.006	3	-.007	15	.702	3
50			min	2.329	15	-196.147	1	.529	12	-.009	1	-.172	1	-.789	1
51		7	max	56.238	1	312.955	3	67.462	1	.006	3	-.005	15	.401	3
52			min	2.329	15	-327.481	1	2.333	12	-.009	1	-.117	1	-.477	1
53		8	max	56.238	1	435.936	3	111.321	1	.006	3	0	10	0	9
54			min	2.329	15	-458.815	1	4.136	12	-.009	1	-.011	1	-.046	3
55		9	max	56.238	1	558.918	3	155.179	1	.006	3	.149	1	.619	1
56			min	2.329	15	-590.149	1	5.94	12	-.009	1	.004	12	-.64	3
57		10	max	56.238	1	681.899	3	199.038	1	.009	1	.36	1	1.403	1
58			min	2.329	15	-721.483	1	7.743	12	-.006	3	.012	12	-1.381	3
59		11	max	56.238	1	590.149	1	-5.94	12	.009	1	.149	1	.619	1
60			min	2.329	15	-558.918	3	-155.179	1	-.006	3	.004	12	-.64	3
61		12	max	56.238	1	458.815	1	-4.136	12	.009	1	0	10	0	9
62			min	2.329	15	-435.936	3	-111.321	1	-.006	3	-.011	1	-.046	3
63		13	max	56.238	1	327.481	1	-2.333	12	.009	1	-.005	15	.401	3
64			min	2.329	15	-312.955	3	-67.462	1	-.006	3	-.117	1	-.477	1
65		14	max	56.238	1	196.147	1	-.529	12	.009	1	-.007	15	.702	3
66			min	2.329	15	-189.974	3	-23.604	1	-.006	3	-.172	1	-.789	1
67		15	max	56.238	1	64.813	1	20.255	1	.009	1	-.007	12	.855	3
68			min	2.329	15	-66.993	3	.844	15	-.006	3	-.174	1	-.945	1
69		16	max	56.238	1	55.988	3	64.114	1	.009	1	-.004	12	.862	3
70			min	2.329	15	-66.522	1	2.647	15	-.006	3	-.123	1	-.944	1
71		17	max	56.238	1	178.969	3	107.972	1	.009	1	0	3	.721	3
72			min	2.329	15	-197.856	1	4.451	15	-.006	3	-.021	1	-.786	1
73		18	max	56.238	1	301.95	3	151.831	1	.009	1	.135	1	.434	3
74			min	2.329	15	-329.19	1	6.255	15	-.006	3	.006	15	-.472	1
75		19	max	56.238	1	424.931	3	195.689	1	.009	1	.342	1	0	1
76			min	2.329	15	-460.524	1	8.058	15	-.006	3	.014	15	0	3
77	M15	1	max	-2.457	15	533.929	2	-8.056	15	.01	1	.342	1	0	2
78			min	-59.341	1	-219.192	3	-195.656	1	-.005	3	.014	15	0	15
79		2	max	-2.457	15	380.495	2	-6.253	15	.01	1	.134	1	.225	3
80			min	-59.341	1	-157.213	3	-151.797	1	-.005	3	.006	15	-.546	2
81		3	max	-2.457	15	227.061	2	-4.449	15	.01	1	0	3	.376	3
82			min	-59.341	1	-95.235	3	-107.939	1	-.005	3	-.021	1	-.909	2
83		4	max	-2.457	15	74.12	1	-2.645	15	.01	1	-.004	12	.452	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-59.341	1	-33.256	3	-64.08	1	-.005	3	-.124	1	-1.089	2
85		5	max	-2.457	15	28.722	3	-.842	15	.01	1	-.007	12	.455	3
86			min	-59.341	1	-79.806	2	-20.222	1	-.005	3	-.174	1	-1.085	2
87		6	max	-2.457	15	90.701	3	23.637	1	.01	1	-.007	15	.384	3
88			min	-59.341	1	-233.24	2	.564	12	-.005	3	-.172	1	-.898	2
89		7	max	-2.457	15	152.679	3	67.496	1	.01	1	-.005	15	.238	3
90			min	-59.341	1	-386.674	2	2.368	12	-.005	3	-.117	1	-.531	1
91		8	max	-2.457	15	214.657	3	111.354	1	.01	1	0	10	.026	2
92			min	-59.341	1	-540.108	2	4.171	12	-.005	3	-.011	1	0	15
93		9	max	-2.457	15	276.636	3	155.213	1	.01	1	.149	1	.763	2
94			min	-59.341	1	-693.541	2	5.974	12	-.005	3	.004	12	-.274	3
95		10	max	-2.457	15	338.614	3	199.071	1	.005	3	.36	1	1.683	2
96			min	-59.341	1	-846.975	2	7.778	12	-.01	1	.013	12	-.642	3
97		11	max	-2.457	15	693.541	2	-5.974	12	.005	3	.149	1	.763	2
98			min	-59.341	1	-276.636	3	-155.213	1	-.01	1	.004	12	-.274	3
99		12	max	-2.457	15	540.108	2	-4.171	12	.005	3	0	10	.026	2
100			min	-59.341	1	-214.657	3	-111.354	1	-.01	1	-.011	1	0	15
101		13	max	-2.457	15	386.674	2	-2.368	12	.005	3	-.005	15	.238	3
102			min	-59.341	1	-152.679	3	-67.496	1	-.01	1	-.117	1	-.531	1
103		14	max	-2.457	15	233.24	2	-.564	12	.005	3	-.007	15	.384	3
104			min	-59.341	1	-90.701	3	-23.637	1	-.01	1	-.172	1	-.898	2
105		15	max	-2.457	15	79.806	2	20.222	1	.005	3	-.007	12	.455	3
106			min	-59.341	1	-28.722	3	.842	15	-.01	1	-.174	1	-1.085	2
107		16	max	-2.457	15	33.256	3	64.08	1	.005	3	-.004	12	.452	3
108			min	-59.341	1	-74.12	1	2.645	15	-.01	1	-.124	1	-1.089	2
109		17	max	-2.457	15	95.235	3	107.939	1	.005	3	0	3	.376	3
110			min	-59.341	1	-227.061	2	4.449	15	-.01	1	-.021	1	-.909	2
111		18	max	-2.457	15	157.213	3	151.797	1	.005	3	.134	1	.225	3
112			min	-59.341	1	-380.495	2	6.253	15	-.01	1	.006	15	-.546	2
113		19	max	-2.457	15	219.192	3	195.656	1	.005	3	.342	1	0	2
114			min	-59.341	1	-533.929	2	8.056	15	-.01	1	.014	15	0	15
115	M16	1	max	-5.586	15	514.57	2	-7.837	15	.01	1	.304	1	0	2
116			min	-134.977	1	-206.369	3	-190.361	1	-.008	3	.013	15	0	3
117		2	max	-5.586	15	361.136	2	-6.033	15	.01	1	.103	1	.209	3
118			min	-134.977	1	-144.39	3	-146.503	1	-.008	3	.004	15	-.523	2
119		3	max	-5.586	15	207.702	2	-4.23	15	.01	1	0	12	.345	3
120			min	-134.977	1	-82.412	3	-102.644	1	-.008	3	-.046	1	-.863	2
121		4	max	-5.586	15	54.269	2	-2.426	15	.01	1	-.005	12	.406	3
122			min	-134.977	1	-20.433	3	-58.785	1	-.008	3	-.143	1	-1.019	2
123		5	max	-5.586	15	41.545	3	-.623	15	.01	1	-.008	12	.394	3
124			min	-134.977	1	-99.165	2	-14.927	1	-.008	3	-.187	1	-.992	2
125		6	max	-5.586	15	103.523	3	28.932	1	.01	1	-.007	15	.307	3
126			min	-134.977	1	-252.599	2	.861	12	-.008	3	-.178	1	-.782	2
127		7	max	-5.586	15	165.502	3	72.791	1	.01	1	-.005	15	.146	3
128			min	-134.977	1	-406.033	2	2.665	12	-.008	3	-.117	1	-.389	2
129		8	max	-5.586	15	227.48	3	116.649	1	.01	1	0	10	.191	1
130			min	-134.977	1	-559.467	2	4.468	12	-.008	3	-.004	1	-.088	3
131		9	max	-5.586	15	289.459	3	160.508	1	.01	1	.161	1	.948	2
132			min	-134.977	1	-712.9	2	6.271	12	-.008	3	.005	12	-.397	3
133		10	max	-5.586	15	351.437	3	204.366	1	.008	3	.379	1	1.891	2
134			min	-134.977	1	-866.334	2	8.075	12	-.01	1	.014	12	-.78	3
135		11	max	-5.586	15	712.9	2	-6.271	12	.008	3	.161	1	.948	2
136			min	-134.977	1	-289.459	3	-160.508	1	-.01	1	.005	12	-.397	3
137		12	max	-5.586	15	559.467	2	-4.468	12	.008	3	0	10	.191	1
138			min	-134.977	1	-227.48	3	-116.649	1	-.01	1	-.004	1	-.088	3
139		13	max	-5.586	15	406.033	2	-2.665	12	.008	3	-.005	15	.146	3
140			min	-134.977	1	-165.502	3	-72.791	1	-.01	1	-.117	1	-.389	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-5.586	15	252.599	2	-.861	12	.008	3	-.007	15	.307	3
142			min	-134.977	1	-103.523	3	-28.932	1	-.01	1	-.178	1	-.782	2
143		15	max	-5.586	15	99.165	2	14.927	1	.008	3	-.008	12	.394	3
144			min	-134.977	1	-41.545	3	.623	15	-.01	1	-.187	1	-.992	2
145		16	max	-5.586	15	20.433	3	58.785	1	.008	3	-.005	12	.406	3
146			min	-134.977	1	-54.269	2	2.426	15	-.01	1	-.143	1	-1.019	2
147		17	max	-5.586	15	82.412	3	102.644	1	.008	3	0	12	.345	3
148			min	-134.977	1	-207.702	2	4.23	15	-.01	1	-.046	1	-.863	2
149		18	max	-5.586	15	144.39	3	146.503	1	.008	3	.103	1	.209	3
150			min	-134.977	1	-361.136	2	6.033	15	-.01	1	.004	15	-.523	2
151		19	max	-5.586	15	206.369	3	190.361	1	.008	3	.304	1	0	2
152			min	-134.977	1	-514.57	2	7.837	15	-.01	1	.013	15	0	3
153	M2	1	max	982.876	1	1.92	4	.814	1	0	5	0	3	0	1
154			min	-993.98	3	.452	15	.034	15	0	1	0	1	0	1
155		2	max	983.304	1	1.863	4	.814	1	0	5	0	1	0	15
156			min	-993.658	3	.439	15	.034	15	0	1	0	15	0	4
157		3	max	983.733	1	1.806	4	.814	1	0	5	0	1	0	15
158			min	-993.337	3	.426	15	.034	15	0	1	0	15	-.001	4
159		4	max	984.161	1	1.75	4	.814	1	0	5	0	1	0	15
160			min	-993.016	3	.412	15	.034	15	0	1	0	15	-.002	4
161		5	max	984.59	1	1.693	4	.814	1	0	5	0	1	0	15
162			min	-992.694	3	.399	15	.034	15	0	1	0	15	-.002	4
163		6	max	985.018	1	1.636	4	.814	1	0	5	.001	1	0	15
164			min	-992.373	3	.385	15	.034	15	0	1	0	15	-.003	4
165		7	max	985.447	1	1.579	4	.814	1	0	5	.001	1	0	15
166			min	-992.052	3	.372	15	.034	15	0	1	0	15	-.003	4
167		8	max	985.875	1	1.523	4	.814	1	0	5	.002	1	0	15
168			min	-991.73	3	.359	15	.034	15	0	1	0	15	-.003	4
169		9	max	986.304	1	1.466	4	.814	1	0	5	.002	1	0	15
170			min	-991.409	3	.345	15	.034	15	0	1	0	15	-.004	4
171		10	max	986.732	1	1.409	4	.814	1	0	5	.002	1	-.001	15
172			min	-991.088	3	.332	15	.034	15	0	1	0	15	-.004	4
173		11	max	987.161	1	1.352	4	.814	1	0	5	.002	1	-.001	15
174			min	-990.766	3	.319	15	.034	15	0	1	0	15	-.005	4
175		12	max	987.589	1	1.295	4	.814	1	0	5	.003	1	-.001	15
176			min	-990.445	3	.305	15	.034	15	0	1	0	15	-.005	4
177		13	max	988.018	1	1.239	4	.814	1	0	5	.003	1	-.001	15
178			min	-990.123	3	.292	15	.034	15	0	1	0	15	-.006	4
179		14	max	988.446	1	1.182	4	.814	1	0	5	.003	1	-.001	15
180			min	-989.802	3	.279	15	.034	15	0	1	0	15	-.006	4
181		15	max	988.875	1	1.125	4	.814	1	0	5	.003	1	-.001	15
182			min	-989.481	3	.265	15	.034	15	0	1	0	15	-.006	4
183		16	max	989.303	1	1.068	4	.814	1	0	5	.004	1	-.002	15
184			min	-989.159	3	.252	15	.034	15	0	1	0	15	-.007	4
185		17	max	989.732	1	1.011	4	.814	1	0	5	.004	1	-.002	15
186			min	-988.838	3	.235	12	.034	15	0	1	0	15	-.007	4
187		18	max	990.16	1	.955	4	.814	1	0	5	.004	1	-.002	15
188			min	-988.517	3	.213	12	.034	15	0	1	0	15	-.007	4
189		19	max	990.588	1	.898	4	.814	1	0	5	.004	1	-.002	15
190			min	-988.195	3	.191	12	.034	15	0	1	0	15	-.007	4
191	M3	1	max	414.35	2	7.881	4	.178	1	0	12	0	1	.007	4
192			min	-557.162	3	1.853	15	.007	15	0	1	0	15	.002	15
193		2	max	414.179	2	7.114	4	.178	1	0	12	0	1	.004	4
194			min	-557.29	3	1.673	15	.007	15	0	1	0	15	0	12
195		3	max	414.009	2	6.347	4	.178	1	0	12	0	1	.002	2
196			min	-557.418	3	1.493	15	.007	15	0	1	0	15	0	3
197		4	max	413.838	2	5.58	4	.178	1	0	12	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-557.545	3	1.312	15	.007	15	0	1	0	15	-.002	3
199		5	max	413.668	2	4.812	4	.178	1	0	12	0	1	0	15
200			min	-557.673	3	1.132	15	.007	15	0	1	0	15	-.003	4
201		6	max	413.498	2	4.045	4	.178	1	0	12	0	1	-.001	15
202			min	-557.801	3	.952	15	.007	15	0	1	0	15	-.005	4
203		7	max	413.327	2	3.278	4	.178	1	0	12	0	1	-.002	15
204			min	-557.929	3	.771	15	.007	15	0	1	0	15	-.007	4
205		8	max	413.157	2	2.511	4	.178	1	0	12	0	1	-.002	15
206			min	-558.056	3	.591	15	.007	15	0	1	0	15	-.008	4
207		9	max	412.987	2	1.744	4	.178	1	0	12	.001	1	-.002	15
208			min	-558.184	3	.41	15	.007	15	0	1	0	15	-.009	4
209		10	max	412.816	2	.976	4	.178	1	0	12	.001	1	-.002	15
210			min	-558.312	3	.23	15	.007	15	0	1	0	15	-.009	4
211		11	max	412.646	2	.292	2	.178	1	0	12	.001	1	-.002	15
212			min	-558.44	3	-.054	3	.007	15	0	1	0	15	-.01	4
213		12	max	412.476	2	-.131	15	.178	1	0	12	.001	1	-.002	15
214			min	-558.567	3	-.558	4	.007	15	0	1	0	15	-.01	4
215		13	max	412.305	2	-.311	15	.178	1	0	12	.001	1	-.002	15
216			min	-558.695	3	-1.325	4	.007	15	0	1	0	15	-.009	4
217		14	max	412.135	2	-.491	15	.178	1	0	12	.001	1	-.002	15
218			min	-558.823	3	-2.093	4	.007	15	0	1	0	15	-.008	4
219		15	max	411.965	2	-.672	15	.178	1	0	12	.001	1	-.002	15
220			min	-558.951	3	-2.86	4	.007	15	0	1	0	15	-.007	4
221		16	max	411.794	2	-.852	15	.178	1	0	12	.002	1	-.001	15
222			min	-559.078	3	-3.627	4	.007	15	0	1	0	15	-.006	4
223		17	max	411.624	2	-1.032	15	.178	1	0	12	.002	1	-.001	15
224			min	-559.206	3	-4.394	4	.007	15	0	1	0	15	-.004	4
225		18	max	411.454	2	-1.213	15	.178	1	0	12	.002	1	0	15
226			min	-559.334	3	-5.161	4	.007	15	0	1	0	15	-.002	4
227		19	max	411.283	2	-1.393	15	.178	1	0	12	.002	1	0	1
228			min	-559.462	3	-5.929	4	.007	15	0	1	0	15	0	1
229	M4	1	max	1130.021	1	0	1	-.527	15	0	1	.001	1	0	1
230			min	-85.419	3	0	1	-12.792	1	0	1	0	15	0	1
231		2	max	1130.191	1	0	1	-.527	15	0	1	0	12	0	1
232			min	-85.292	3	0	1	-12.792	1	0	1	0	1	0	1
233		3	max	1130.361	1	0	1	-.527	15	0	1	0	15	0	1
234			min	-85.164	3	0	1	-12.792	1	0	1	-.002	1	0	1
235		4	max	1130.532	1	0	1	-.527	15	0	1	0	15	0	1
236			min	-85.036	3	0	1	-12.792	1	0	1	-.003	1	0	1
237		5	max	1130.702	1	0	1	-.527	15	0	1	0	15	0	1
238			min	-84.908	3	0	1	-12.792	1	0	1	-.005	1	0	1
239		6	max	1130.872	1	0	1	-.527	15	0	1	0	15	0	1
240			min	-84.781	3	0	1	-12.792	1	0	1	-.006	1	0	1
241		7	max	1131.043	1	0	1	-.527	15	0	1	0	15	0	1
242			min	-84.653	3	0	1	-12.792	1	0	1	-.008	1	0	1
243		8	max	1131.213	1	0	1	-.527	15	0	1	0	15	0	1
244			min	-84.525	3	0	1	-12.792	1	0	1	-.009	1	0	1
245		9	max	1131.383	1	0	1	-.527	15	0	1	0	15	0	1
246			min	-84.397	3	0	1	-12.792	1	0	1	-.01	1	0	1
247		10	max	1131.554	1	0	1	-.527	15	0	1	0	15	0	1
248			min	-84.269	3	0	1	-12.792	1	0	1	-.012	1	0	1
249		11	max	1131.724	1	0	1	-.527	15	0	1	0	15	0	1
250			min	-84.142	3	0	1	-12.792	1	0	1	-.013	1	0	1
251		12	max	1131.894	1	0	1	-.527	15	0	1	0	15	0	1
252			min	-84.014	3	0	1	-12.792	1	0	1	-.015	1	0	1
253		13	max	1132.065	1	0	1	-.527	15	0	1	0	15	0	1
254			min	-83.886	3	0	1	-12.792	1	0	1	-.016	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1132.235	1	0	1	-527	15	0	1	0	15	0	1
256		min	-83.758	3	0	1	-12.792	1	0	1	-.018	1	0	1
257	15	max	1132.405	1	0	1	-527	15	0	1	0	15	0	1
258		min	-83.631	3	0	1	-12.792	1	0	1	-.019	1	0	1
259	16	max	1132.576	1	0	1	-527	15	0	1	0	15	0	1
260		min	-83.503	3	0	1	-12.792	1	0	1	-.021	1	0	1
261	17	max	1132.746	1	0	1	-527	15	0	1	0	15	0	1
262		min	-83.375	3	0	1	-12.792	1	0	1	-.022	1	0	1
263	18	max	1132.916	1	0	1	-527	15	0	1	0	15	0	1
264		min	-83.247	3	0	1	-12.792	1	0	1	-.024	1	0	1
265	19	max	1133.087	1	0	1	-527	15	0	1	-.001	15	0	1
266		min	-83.12	3	0	1	-12.792	1	0	1	-.025	1	0	1
267	M6	1	max	3176.179	1	2.193	2	0	1	0	1	0	1	1
268		min	-3269.109	3	.235	12	0	1	0	1	0	1	0	1
269	2	max	3176.607	1	2.149	2	0	1	0	1	0	1	0	12
270		min	-3268.788	3	.213	12	0	1	0	1	0	1	0	2
271	3	max	3177.036	1	2.104	2	0	1	0	1	0	1	0	12
272		min	-3268.466	3	.191	12	0	1	0	1	0	1	-.001	2
273	4	max	3177.464	1	2.06	2	0	1	0	1	0	1	0	12
274		min	-3268.145	3	.169	12	0	1	0	1	0	1	-.002	2
275	5	max	3177.893	1	2.016	2	0	1	0	1	0	1	0	12
276		min	-3267.824	3	.147	12	0	1	0	1	0	1	-.002	2
277	6	max	3178.321	1	1.972	2	0	1	0	1	0	1	0	12
278		min	-3267.502	3	.125	12	0	1	0	1	0	1	-.003	2
279	7	max	3178.75	1	1.927	2	0	1	0	1	0	1	0	12
280		min	-3267.181	3	.102	3	0	1	0	1	0	1	-.004	2
281	8	max	3179.178	1	1.883	2	0	1	0	1	0	1	0	12
282		min	-3266.86	3	.068	3	0	1	0	1	0	1	-.004	2
283	9	max	3179.607	1	1.839	2	0	1	0	1	0	1	0	12
284		min	-3266.538	3	.035	3	0	1	0	1	0	1	-.005	2
285	10	max	3180.035	1	1.795	2	0	1	0	1	0	1	0	12
286		min	-3266.217	3	.002	3	0	1	0	1	0	1	-.005	2
287	11	max	3180.464	1	1.751	2	0	1	0	1	0	1	0	12
288		min	-3265.895	3	-.031	3	0	1	0	1	0	1	-.006	2
289	12	max	3180.892	1	1.706	2	0	1	0	1	0	1	0	12
290		min	-3265.574	3	-.064	3	0	1	0	1	0	1	-.006	2
291	13	max	3181.321	1	1.662	2	0	1	0	1	0	1	0	3
292		min	-3265.253	3	-.098	3	0	1	0	1	0	1	-.007	2
293	14	max	3181.749	1	1.618	2	0	1	0	1	0	1	0	3
294		min	-3264.931	3	-.131	3	0	1	0	1	0	1	-.007	2
295	15	max	3182.178	1	1.574	2	0	1	0	1	0	1	0	3
296		min	-3264.61	3	-.164	3	0	1	0	1	0	1	-.008	2
297	16	max	3182.606	1	1.529	2	0	1	0	1	0	1	0	3
298		min	-3264.289	3	-.197	3	0	1	0	1	0	1	-.008	2
299	17	max	3183.035	1	1.485	2	0	1	0	1	0	1	0	3
300		min	-3263.967	3	-.23	3	0	1	0	1	0	1	-.009	2
301	18	max	3183.463	1	1.441	2	0	1	0	1	0	1	0	3
302		min	-3263.646	3	-.263	3	0	1	0	1	0	1	-.009	2
303	19	max	3183.892	1	1.397	2	0	1	0	1	0	1	0	3
304		min	-3263.325	3	-.297	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	1692.724	2	7.92	4	0	1	0	1	0	.009	2
306		min	-1751.82	3	1.859	15	0	1	0	1	0	1	0	3
307	2	max	1692.554	2	7.153	4	0	1	0	1	0	1	.007	2
308		min	-1751.948	3	1.679	15	0	1	0	1	0	1	-.002	3
309	3	max	1692.383	2	6.386	4	0	1	0	1	0	1	.004	2
310		min	-1752.075	3	1.498	15	0	1	0	1	0	1	-.003	3
311	4	max	1692.213	2	5.618	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-1752.203	3	1.318	15	0	1	0	1	0	1	-.004	3
313	5	max	1692.043	2	4.851	4	0	1	0	1	0	1	0	2
314		min	-1752.331	3	1.137	15	0	1	0	1	0	1	-.005	3
315	6	max	1691.872	2	4.084	4	0	1	0	1	0	1	-.001	15
316		min	-1752.459	3	.957	15	0	1	0	1	0	1	-.006	3
317	7	max	1691.702	2	3.317	4	0	1	0	1	0	1	-.002	15
318		min	-1752.586	3	.777	15	0	1	0	1	0	1	-.007	3
319	8	max	1691.531	2	2.55	4	0	1	0	1	0	1	-.002	15
320		min	-1752.714	3	.596	15	0	1	0	1	0	1	-.008	4
321	9	max	1691.361	2	1.842	2	0	1	0	1	0	1	-.002	15
322		min	-1752.842	3	.324	12	0	1	0	1	0	1	-.009	4
323	10	max	1691.191	2	1.244	2	0	1	0	1	0	1	-.002	15
324		min	-1752.97	3	.001	3	0	1	0	1	0	1	-.009	4
325	11	max	1691.02	2	.646	2	0	1	0	1	0	1	-.002	15
326		min	-1753.097	3	-.447	3	0	1	0	1	0	1	-.009	4
327	12	max	1690.85	2	.048	2	0	1	0	1	0	1	-.002	15
328		min	-1753.225	3	-.895	3	0	1	0	1	0	1	-.009	4
329	13	max	1690.68	2	-.305	15	0	1	0	1	0	1	-.002	15
330		min	-1753.353	3	-1.344	3	0	1	0	1	0	1	-.009	4
331	14	max	1690.509	2	-.486	15	0	1	0	1	0	1	-.002	15
332		min	-1753.481	3	-2.054	4	0	1	0	1	0	1	-.008	4
333	15	max	1690.339	2	-.666	15	0	1	0	1	0	1	-.002	15
334		min	-1753.608	3	-2.821	4	0	1	0	1	0	1	-.007	4
335	16	max	1690.169	2	-.846	15	0	1	0	1	0	1	-.001	15
336		min	-1753.736	3	-3.588	4	0	1	0	1	0	1	-.006	4
337	17	max	1689.998	2	-1.027	15	0	1	0	1	0	1	-.001	15
338		min	-1753.864	3	-4.355	4	0	1	0	1	0	1	-.004	4
339	18	max	1689.828	2	-1.207	15	0	1	0	1	0	1	0	15
340		min	-1753.992	3	-5.123	4	0	1	0	1	0	1	-.002	4
341	19	max	1689.658	2	-1.387	15	0	1	0	1	0	1	0	1
342		min	-1754.119	3	-5.89	4	0	1	0	1	0	1	0	1
343	M8	1	max	3025.401	1	0	1	0	1	0	1	0	1	1
344		min	-359.57	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3025.572	1	0	1	0	1	0	1	0	1	0	1
346		min	-359.442	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3025.742	1	0	1	0	1	0	1	0	1	0	1
348		min	-359.314	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3025.912	1	0	1	0	1	0	1	0	1	0	1
350		min	-359.187	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3026.083	1	0	1	0	1	0	1	0	1	0	1
352		min	-359.059	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3026.253	1	0	1	0	1	0	1	0	1	0	1
354		min	-358.931	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3026.423	1	0	1	0	1	0	1	0	1	0	1
356		min	-358.803	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3026.594	1	0	1	0	1	0	1	0	1	0	1
358		min	-358.676	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3026.764	1	0	1	0	1	0	1	0	1	0	1
360		min	-358.548	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3026.934	1	0	1	0	1	0	1	0	1	0	1
362		min	-358.42	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3027.105	1	0	1	0	1	0	1	0	1	0	1
364		min	-358.292	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3027.275	1	0	1	0	1	0	1	0	1	0	1
366		min	-358.165	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3027.445	1	0	1	0	1	0	1	0	1	0	1
368		min	-358.037	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3027.616	1	0	1	0	1	0	1	0	1	0	1
370			min	-357.909	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3027.786	1	0	1	0	1	0	1	0	1	0	1
372			min	-357.781	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3027.956	1	0	1	0	1	0	1	0	1	0	1
374			min	-357.654	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3028.127	1	0	1	0	1	0	1	0	1	0	1
376			min	-357.526	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3028.297	1	0	1	0	1	0	1	0	1	0	1
378			min	-357.398	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3028.468	1	0	1	0	1	0	1	0	1	0	1
380			min	-357.27	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	982.876	1	1.92	4	-.034	15	0	1	0	1	0	1
382			min	-993.98	3	.452	15	-.814	1	0	5	0	3	0	1
383		2	max	983.304	1	1.863	4	-.034	15	0	1	0	15	0	15
384			min	-993.658	3	.439	15	-.814	1	0	5	0	1	0	4
385		3	max	983.733	1	1.806	4	-.034	15	0	1	0	15	0	15
386			min	-993.337	3	.426	15	-.814	1	0	5	0	1	-.001	4
387		4	max	984.161	1	1.75	4	-.034	15	0	1	0	15	0	15
388			min	-993.016	3	.412	15	-.814	1	0	5	0	1	-.002	4
389		5	max	984.59	1	1.693	4	-.034	15	0	1	0	15	0	15
390			min	-992.694	3	.399	15	-.814	1	0	5	0	1	-.002	4
391		6	max	985.018	1	1.636	4	-.034	15	0	1	0	15	0	15
392			min	-992.373	3	.385	15	-.814	1	0	5	-.001	1	-.003	4
393		7	max	985.447	1	1.579	4	-.034	15	0	1	0	15	0	15
394			min	-992.052	3	.372	15	-.814	1	0	5	-.001	1	-.003	4
395		8	max	985.875	1	1.523	4	-.034	15	0	1	0	15	0	15
396			min	-991.73	3	.359	15	-.814	1	0	5	-.002	1	-.003	4
397		9	max	986.304	1	1.466	4	-.034	15	0	1	0	15	0	15
398			min	-991.409	3	.345	15	-.814	1	0	5	-.002	1	-.004	4
399		10	max	986.732	1	1.409	4	-.034	15	0	1	0	15	-.001	15
400			min	-991.088	3	.332	15	-.814	1	0	5	-.002	1	-.004	4
401		11	max	987.161	1	1.352	4	-.034	15	0	1	0	15	-.001	15
402			min	-990.766	3	.319	15	-.814	1	0	5	-.002	1	-.005	4
403		12	max	987.589	1	1.295	4	-.034	15	0	1	0	15	-.001	15
404			min	-990.445	3	.305	15	-.814	1	0	5	-.003	1	-.005	4
405		13	max	988.018	1	1.239	4	-.034	15	0	1	0	15	-.001	15
406			min	-990.123	3	.292	15	-.814	1	0	5	-.003	1	-.006	4
407		14	max	988.446	1	1.182	4	-.034	15	0	1	0	15	-.001	15
408			min	-989.802	3	.279	15	-.814	1	0	5	-.003	1	-.006	4
409		15	max	988.875	1	1.125	4	-.034	15	0	1	0	15	-.001	15
410			min	-989.481	3	.265	15	-.814	1	0	5	-.003	1	-.006	4
411		16	max	989.303	1	1.068	4	-.034	15	0	1	0	15	-.002	15
412			min	-989.159	3	.252	15	-.814	1	0	5	-.004	1	-.007	4
413		17	max	989.732	1	1.011	4	-.034	15	0	1	0	15	-.002	15
414			min	-988.838	3	.235	12	-.814	1	0	5	-.004	1	-.007	4
415		18	max	990.16	1	.955	4	-.034	15	0	1	0	15	-.002	15
416			min	-988.517	3	.213	12	-.814	1	0	5	-.004	1	-.007	4
417		19	max	990.588	1	.898	4	-.034	15	0	1	0	15	-.002	15
418			min	-988.195	3	.191	12	-.814	1	0	5	-.004	1	-.007	4
419	M11	1	max	414.35	2	7.881	4	-.007	15	0	1	0	15	.007	4
420			min	-557.162	3	1.853	15	-.178	1	0	12	0	1	.002	15
421		2	max	414.179	2	7.114	4	-.007	15	0	1	0	15	.004	4
422			min	-557.29	3	1.673	15	-.178	1	0	12	0	1	0	12
423		3	max	414.009	2	6.347	4	-.007	15	0	1	0	15	.002	2
424			min	-557.418	3	1.493	15	-.178	1	0	12	0	1	0	3
425		4	max	413.838	2	5.58	4	-.007	15	0	1	0	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-557.545	3	1.312	15	-.178	1	0	12	0	1	-.002	3
427		5	max	413.668	2	4.812	4	-.007	15	0	1	0	15	0	15
428			min	-557.673	3	1.132	15	-.178	1	0	12	0	1	-.003	4
429		6	max	413.498	2	4.045	4	-.007	15	0	1	0	15	-.001	15
430			min	-557.801	3	.952	15	-.178	1	0	12	0	1	-.005	4
431		7	max	413.327	2	3.278	4	-.007	15	0	1	0	15	-.002	15
432			min	-557.929	3	.771	15	-.178	1	0	12	0	1	-.007	4
433		8	max	413.157	2	2.511	4	-.007	15	0	1	0	15	-.002	15
434			min	-558.056	3	.591	15	-.178	1	0	12	0	1	-.008	4
435		9	max	412.987	2	1.744	4	-.007	15	0	1	0	15	-.002	15
436			min	-558.184	3	.41	15	-.178	1	0	12	-.001	1	-.009	4
437		10	max	412.816	2	.976	4	-.007	15	0	1	0	15	-.002	15
438			min	-558.312	3	.23	15	-.178	1	0	12	-.001	1	-.009	4
439		11	max	412.646	2	.292	2	-.007	15	0	1	0	15	-.002	15
440			min	-558.44	3	-.054	3	-.178	1	0	12	-.001	1	-.01	4
441		12	max	412.476	2	-.131	15	-.007	15	0	1	0	15	-.002	15
442			min	-558.567	3	-.558	4	-.178	1	0	12	-.001	1	-.01	4
443		13	max	412.305	2	-.311	15	-.007	15	0	1	0	15	-.002	15
444			min	-558.695	3	-1.325	4	-.178	1	0	12	-.001	1	-.009	4
445		14	max	412.135	2	-.491	15	-.007	15	0	1	0	15	-.002	15
446			min	-558.823	3	-2.093	4	-.178	1	0	12	-.001	1	-.008	4
447		15	max	411.965	2	-.672	15	-.007	15	0	1	0	15	-.002	15
448			min	-558.951	3	-2.86	4	-.178	1	0	12	-.001	1	-.007	4
449		16	max	411.794	2	-.852	15	-.007	15	0	1	0	15	-.001	15
450			min	-559.078	3	-3.627	4	-.178	1	0	12	-.002	1	-.006	4
451		17	max	411.624	2	-1.032	15	-.007	15	0	1	0	15	-.001	15
452			min	-559.206	3	-4.394	4	-.178	1	0	12	-.002	1	-.004	4
453		18	max	411.454	2	-1.213	15	-.007	15	0	1	0	15	0	15
454			min	-559.334	3	-5.161	4	-.178	1	0	12	-.002	1	-.002	4
455		19	max	411.283	2	-1.393	15	-.007	15	0	1	0	15	0	1
456			min	-559.462	3	-5.929	4	-.178	1	0	12	-.002	1	0	1
457	M12	1	max	1130.021	1	0	1	12.792	1	0	1	0	15	0	1
458			min	-85.419	3	0	1	.527	15	0	1	-.001	1	0	1
459		2	max	1130.191	1	0	1	12.792	1	0	1	0	1	0	1
460			min	-85.292	3	0	1	.527	15	0	1	0	12	0	1
461		3	max	1130.361	1	0	1	12.792	1	0	1	.002	1	0	1
462			min	-85.164	3	0	1	.527	15	0	1	0	15	0	1
463		4	max	1130.532	1	0	1	12.792	1	0	1	.003	1	0	1
464			min	-85.036	3	0	1	.527	15	0	1	0	15	0	1
465		5	max	1130.702	1	0	1	12.792	1	0	1	.005	1	0	1
466			min	-84.908	3	0	1	.527	15	0	1	0	15	0	1
467		6	max	1130.872	1	0	1	12.792	1	0	1	.006	1	0	1
468			min	-84.781	3	0	1	.527	15	0	1	0	15	0	1
469		7	max	1131.043	1	0	1	12.792	1	0	1	.008	1	0	1
470			min	-84.653	3	0	1	.527	15	0	1	0	15	0	1
471		8	max	1131.213	1	0	1	12.792	1	0	1	.009	1	0	1
472			min	-84.525	3	0	1	.527	15	0	1	0	15	0	1
473		9	max	1131.383	1	0	1	12.792	1	0	1	.01	1	0	1
474			min	-84.397	3	0	1	.527	15	0	1	0	15	0	1
475		10	max	1131.554	1	0	1	12.792	1	0	1	.012	1	0	1
476			min	-84.269	3	0	1	.527	15	0	1	0	15	0	1
477		11	max	1131.724	1	0	1	12.792	1	0	1	.013	1	0	1
478			min	-84.142	3	0	1	.527	15	0	1	0	15	0	1
479		12	max	1131.894	1	0	1	12.792	1	0	1	.015	1	0	1
480			min	-84.014	3	0	1	.527	15	0	1	0	15	0	1
481		13	max	1132.065	1	0	1	12.792	1	0	1	.016	1	0	1
482			min	-83.886	3	0	1	.527	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	1132.235	1	0	1	12.792	1	0	1	.018	1	0	1
484			min	-83.758	3	0	1	.527	15	0	1	0	15	0	1
485		15	max	1132.405	1	0	1	12.792	1	0	1	.019	1	0	1
486			min	-83.631	3	0	1	.527	15	0	1	0	15	0	1
487		16	max	1132.576	1	0	1	12.792	1	0	1	.021	1	0	1
488			min	-83.503	3	0	1	.527	15	0	1	0	15	0	1
489		17	max	1132.746	1	0	1	12.792	1	0	1	.022	1	0	1
490			min	-83.375	3	0	1	.527	15	0	1	0	15	0	1
491		18	max	1132.916	1	0	1	12.792	1	0	1	.024	1	0	1
492			min	-83.247	3	0	1	.527	15	0	1	0	15	0	1
493		19	max	1133.087	1	0	1	12.792	1	0	1	.025	1	0	1
494			min	-83.12	3	0	1	.527	15	0	1	.001	15	0	1
495	M1	1	max	190.144	1	552.523	3	-5.228	15	0	1	.302	1	0	3
496			min	7.83	15	-437.899	1	-126.383	1	0	3	.012	15	-.012	1
497		2	max	190.75	1	551.549	3	-5.228	15	0	1	.236	1	.22	1
498			min	8.013	15	-439.197	1	-126.383	1	0	3	.01	15	-.291	3
499		3	max	339.358	3	486.102	1	-5.195	15	0	3	.169	1	.441	1
500			min	-203.848	2	-389.912	3	-125.852	1	0	1	.007	15	-.571	3
501		4	max	339.812	3	484.804	1	-5.195	15	0	3	.103	1	.185	1
502			min	-203.242	2	-390.885	3	-125.852	1	0	1	.004	15	-.365	3
503		5	max	340.266	3	483.505	1	-5.195	15	0	3	.036	1	-.003	15
504			min	-202.637	2	-391.859	3	-125.852	1	0	1	.001	15	-.158	3
505		6	max	340.72	3	482.207	1	-5.195	15	0	3	-.001	15	.049	3
506			min	-202.032	2	-392.833	3	-125.852	1	0	1	-.03	1	-.326	1
507		7	max	341.174	3	480.909	1	-5.195	15	0	3	-.004	15	.256	3
508			min	-201.426	2	-393.806	3	-125.852	1	0	1	-.097	1	-.58	1
509		8	max	341.628	3	479.611	1	-5.195	15	0	3	-.007	15	.465	3
510			min	-200.821	2	-394.78	3	-125.852	1	0	1	-.163	1	-.833	1
511		9	max	354.855	3	35.056	2	-7.524	15	0	9	.095	1	.544	3
512			min	-121.142	2	.396	15	-182.125	1	0	3	.004	15	-.95	1
513		10	max	355.309	3	33.758	2	-7.524	15	0	9	0	15	.528	3
514			min	-120.537	2	.004	15	-182.125	1	0	3	-.001	1	-.959	1
515		11	max	355.763	3	32.46	2	-7.524	15	0	9	-.004	15	.513	3
516			min	-119.931	2	-1.58	4	-182.125	1	0	3	-.097	1	-.968	1
517		12	max	368.936	3	248.702	3	-5.067	15	0	2	.161	1	.446	3
518			min	-74.604	10	-516.534	1	-122.866	1	0	3	.007	15	-.855	1
519		13	max	369.39	3	247.728	3	-5.067	15	0	2	.096	1	.315	3
520			min	-74.099	10	-517.832	1	-122.866	1	0	3	.004	15	-.582	1
521		14	max	369.844	3	246.755	3	-5.067	15	0	2	.031	1	.185	3
522			min	-73.595	10	-519.13	1	-122.866	1	0	3	.001	15	-.309	1
523		15	max	370.298	3	245.781	3	-5.067	15	0	2	-.001	15	.055	3
524			min	-73.09	10	-520.428	1	-122.866	1	0	3	-.034	1	-.034	1
525		16	max	370.752	3	244.807	3	-5.067	15	0	2	-.004	15	.255	2
526			min	-72.586	10	-521.726	1	-122.866	1	0	3	-.098	1	-.075	3
527		17	max	371.206	3	243.834	3	-5.067	15	0	2	-.007	15	.524	2
528			min	-72.081	10	-523.025	1	-122.866	1	0	3	-.163	1	-.204	3
529		18	max	-8.02	15	516.348	2	-5.586	15	0	3	-.01	15	.263	2
530			min	-190.962	1	-205.445	3	-135.14	1	0	2	-.233	1	-.101	3
531		19	max	-7.837	15	515.049	2	-5.586	15	0	3	-.013	15	.008	3
532			min	-190.357	1	-206.419	3	-135.14	1	0	2	-.304	1	-.01	1
533	M5	1	max	409.166	1	1840.545	3	0	1	0	1	0	1	.023	1
534			min	15.927	12	-1476.968	1	0	1	0	1	0	1	0	3
535		2	max	409.771	1	1839.572	3	0	1	0	1	0	1	.803	1
536			min	16.229	12	-1478.266	1	0	1	0	1	0	1	-.971	3
537		3	max	1093.595	3	1498.387	1	0	1	0	1	0	1	1.547	1
538			min	-744.486	2	-1265.234	3	0	1	0	1	0	1	-1.904	3
539		4	max	1094.049	3	1497.089	1	0	1	0	1	0	1	.757	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-743.881	2	-1266.208	3	0	1	0	1	0	1	-1.236	3
541		5	max	1094.503	3	1495.791	1	0	1	0	1	0	1	.005	9
542			min	-743.276	2	-1267.182	3	0	1	0	1	0	1	-.568	3
543		6	max	1094.957	3	1494.493	1	0	1	0	1	0	1	.101	3
544			min	-742.67	2	-1268.155	3	0	1	0	1	0	1	-.822	1
545		7	max	1095.411	3	1493.195	1	0	1	0	1	0	1	.77	3
546			min	-742.065	2	-1269.129	3	0	1	0	1	0	1	-1.61	1
547		8	max	1095.865	3	1491.896	1	0	1	0	1	0	1	1.44	3
548			min	-741.459	2	-1270.103	3	0	1	0	1	0	1	-2.397	1
549		9	max	1119.463	3	116.143	2	0	1	0	1	0	1	1.661	3
550			min	-578.308	2	.393	15	0	1	0	1	0	1	-2.713	1
551		10	max	1119.917	3	114.845	2	0	1	0	1	0	1	1.605	3
552			min	-577.702	2	.002	15	0	1	0	1	0	1	-2.745	1
553		11	max	1120.371	3	113.547	2	0	1	0	1	0	1	1.55	3
554			min	-577.097	2	-1.408	4	0	1	0	1	0	1	-2.777	1
555		12	max	1144.079	3	797.647	3	0	1	0	1	0	1	1.359	3
556			min	-413.958	2	-1616.853	1	0	1	0	1	0	1	-2.475	1
557		13	max	1144.533	3	796.673	3	0	1	0	1	0	1	.939	3
558			min	-413.353	2	-1618.151	1	0	1	0	1	0	1	-1.622	1
559		14	max	1144.987	3	795.699	3	0	1	0	1	0	1	.519	3
560			min	-412.747	2	-1619.45	1	0	1	0	1	0	1	-.768	1
561		15	max	1145.441	3	794.726	3	0	1	0	1	0	1	.146	2
562			min	-412.142	2	-1620.748	1	0	1	0	1	0	1	-.004	13
563		16	max	1145.895	3	793.752	3	0	1	0	1	0	1	.978	2
564			min	-411.537	2	-1622.046	1	0	1	0	1	0	1	-.32	3
565		17	max	1146.349	3	792.778	3	0	1	0	1	0	1	1.811	2
566			min	-410.931	2	-1623.344	1	0	1	0	1	0	1	-.739	3
567		18	max	-16.452	12	1736.801	2	0	1	0	1	0	1	.934	2
568			min	-409.347	1	-702.22	3	0	1	0	1	0	1	-.386	3
569		19	max	-16.149	12	1735.502	2	0	1	0	1	0	1	.021	1
570			min	-408.741	1	-703.194	3	0	1	0	1	0	1	-.016	3
571	M9	1	max	190.144	1	552.523	3	126.383	1	0	3	-.012	15	0	3
572			min	7.83	15	-437.899	1	5.228	15	0	1	-.302	1	-.012	1
573		2	max	190.75	1	551.549	3	126.383	1	0	3	-.01	15	.22	1
574			min	8.013	15	-439.197	1	5.228	15	0	1	-.236	1	-.291	3
575		3	max	339.358	3	486.102	1	125.852	1	0	1	-.007	15	.441	1
576			min	-203.848	2	-389.912	3	5.195	15	0	3	-.169	1	-.571	3
577		4	max	339.812	3	484.804	1	125.852	1	0	1	-.004	15	.185	1
578			min	-203.242	2	-390.885	3	5.195	15	0	3	-.103	1	-.365	3
579		5	max	340.266	3	483.505	1	125.852	1	0	1	-.001	15	-.003	15
580			min	-202.637	2	-391.859	3	5.195	15	0	3	-.036	1	-.158	3
581		6	max	340.72	3	482.207	1	125.852	1	0	1	.03	1	.049	3
582			min	-202.032	2	-392.833	3	5.195	15	0	3	.001	15	-.326	1
583		7	max	341.174	3	480.909	1	125.852	1	0	1	.097	1	.256	3
584			min	-201.426	2	-393.806	3	5.195	15	0	3	.004	15	-.58	1
585		8	max	341.628	3	479.611	1	125.852	1	0	1	.163	1	.465	3
586			min	-200.821	2	-394.78	3	5.195	15	0	3	.007	15	-.833	1
587		9	max	354.855	3	35.056	2	182.125	1	0	3	-.004	15	.544	3
588			min	-121.142	2	.396	15	7.524	15	0	9	-.095	1	-.95	1
589		10	max	355.309	3	33.758	2	182.125	1	0	3	.001	1	.528	3
590			min	-120.537	2	.004	15	7.524	15	0	9	0	15	-.959	1
591		11	max	355.763	3	32.46	2	182.125	1	0	3	.097	1	.513	3
592			min	-119.931	2	-1.58	4	7.524	15	0	9	.004	15	-.968	1
593		12	max	368.936	3	248.702	3	122.866	1	0	3	-.007	15	.446	3
594			min	-74.604	10	-516.534	1	5.067	15	0	2	-.161	1	-.855	1
595		13	max	369.39	3	247.728	3	122.866	1	0	3	-.004	15	.315	3
596			min	-74.099	10	-517.832	1	5.067	15	0	2	-.096	1	-.582	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	369.844	3	246.755	3	122.866	1	0	3	-.001	15	.185	3
598		min	-73.595	10	-519.13	1	5.067	15	0	2	-.031	1	-.309	1
599	15	max	370.298	3	245.781	3	122.866	1	0	3	.034	1	.055	3
600		min	-73.09	10	-520.428	1	5.067	15	0	2	.001	15	-.034	1
601	16	max	370.752	3	244.807	3	122.866	1	0	3	.098	1	.255	2
602		min	-72.586	10	-521.726	1	5.067	15	0	2	.004	15	-.075	3
603	17	max	371.206	3	243.834	3	122.866	1	0	3	.163	1	.524	2
604		min	-72.081	10	-523.025	1	5.067	15	0	2	.007	15	-.204	3
605	18	max	-8.02	15	516.348	2	135.14	1	0	2	.233	1	.263	2
606		min	-190.962	1	-205.445	3	5.586	15	0	3	.01	15	-.101	3
607	19	max	-7.837	15	515.049	2	135.14	1	0	2	.304	1	.008	3
608		min	-190.357	1	-206.419	3	5.586	15	0	3	.013	15	-.01	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.095	1	.006	3	7.624e-3	1	NC	1	NC	1
2			min	0	15	-.009	3	-.003	2	-8.47e-4	3	NC	1	NC	1
3		2	max	.001	1	.317	3	.054	1	8.837e-3	1	NC	5	NC	2
4			min	0	15	-.144	1	.002	15	-8.602e-4	3	793.292	3	5001.964	1
5		3	max	.001	1	.58	3	.129	1	1.005e-2	1	NC	5	NC	3
6			min	0	15	-.333	1	.005	15	-8.734e-4	3	438.295	3	2027.648	1
7		4	max	0	1	.74	3	.195	1	1.126e-2	1	NC	5	NC	3
8			min	0	15	-.441	1	.008	15	-8.866e-4	3	344.584	3	1335.437	1
9		5	max	0	1	.777	3	.23	1	1.247e-2	1	NC	5	NC	3
10			min	0	15	-.452	1	.01	15	-8.999e-4	3	328.208	3	1133.713	1
11		6	max	0	1	.695	3	.223	1	1.369e-2	1	NC	5	NC	3
12			min	0	15	-.368	1	.009	15	-9.131e-4	3	366.763	3	1170.525	1
13		7	max	0	1	.517	3	.176	1	1.49e-2	1	NC	5	NC	3
14			min	0	15	-.211	1	.007	15	-9.263e-4	3	490.781	3	1485.428	1
15		8	max	0	1	.291	3	.103	1	1.611e-2	1	NC	4	NC	3
16			min	0	15	-.019	1	.004	10	-9.395e-4	3	860.561	3	2547.604	1
17		9	max	0	1	.154	2	.031	1	1.732e-2	1	NC	4	NC	2
18			min	0	15	.005	15	-.004	10	-9.527e-4	3	2718.241	3	8907.538	1
19		10	max	0	1	.229	1	.019	3	1.854e-2	1	NC	3	NC	1
20		min	0	1	-.006	3	-.012	2	-9.66e-4	3	1921.149	1	NC	1	
21	11	max	0	15	.154	2	.031	1	1.732e-2	1	NC	4	NC	2	
22		min	0	1	.005	15	-.004	10	-9.527e-4	3	2718.241	3	8907.538	1	
23	12	max	0	15	.291	3	.103	1	1.611e-2	1	NC	4	NC	3	
24		min	0	1	-.019	1	.004	10	-9.395e-4	3	860.561	3	2547.604	1	
25	13	max	0	15	.517	3	.176	1	1.49e-2	1	NC	5	NC	3	
26		min	0	1	-.211	1	.007	15	-9.263e-4	3	490.781	3	1485.428	1	
27	14	max	0	15	.695	3	.223	1	1.369e-2	1	NC	5	NC	3	
28		min	0	1	-.368	1	.009	15	-9.131e-4	3	366.763	3	1170.525	1	
29	15	max	0	15	.777	3	.23	1	1.247e-2	1	NC	5	NC	3	
30		min	0	1	-.452	1	.01	15	-8.999e-4	3	328.208	3	1133.713	1	
31	16	max	0	15	.74	3	.195	1	1.126e-2	1	NC	5	NC	3	
32		min	0	1	-.441	1	.008	15	-8.866e-4	3	344.584	3	1335.437	1	
33	17	max	0	15	.58	3	.129	1	1.005e-2	1	NC	5	NC	3	
34		min	-.001	1	-.333	1	.005	15	-8.734e-4	3	438.295	3	2027.648	1	
35	18	max	0	15	.317	3	.054	1	8.837e-3	1	NC	5	NC	2	
36		min	-.001	1	-.144	1	.002	15	-8.602e-4	3	793.292	3	5001.964	1	
37	19	max	0	15	.095	1	.006	3	7.624e-3	1	NC	1	NC	1	
38		min	-.001	1	-.009	3	-.003	2	-8.47e-4	3	NC	1	NC	1	
39	M14	1	max	0	1	.16	3	.005	3	4.783e-3	1	NC	1	NC	1
40			min	0	15	-.312	1	-.002	2	-2.9e-3	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.458	3	.038	1	5.77e-3	1	NC	5	NC	2
42			min	0	15	-.666	1	.002	15	-3.554e-3	3	728.223	1	7239.7	1
43		3	max	0	1	.707	3	.105	1	6.756e-3	1	NC	15	NC	3
44			min	0	15	-.968	1	.004	15	-4.208e-3	3	392.898	1	2507.754	1
45		4	max	0	1	.877	3	.169	1	7.742e-3	1	NC	15	NC	3
46			min	0	15	-1.183	1	.007	15	-4.862e-3	3	296.049	1	1550.151	1
47		5	max	0	1	.95	3	.205	1	8.729e-3	1	9139.766	15	NC	3
48			min	0	15	-1.293	1	.009	15	-5.516e-3	3	263.032	1	1271.431	1
49		6	max	0	1	.928	3	.203	1	9.715e-3	1	9145.846	15	NC	3
50			min	0	15	-1.296	1	.009	15	-6.17e-3	3	262.004	1	1284.38	1
51		7	max	0	1	.827	3	.163	1	1.07e-2	1	NC	15	NC	3
52			min	0	15	-1.212	1	.007	15	-6.824e-3	3	286.454	1	1604.947	1
53		8	max	0	1	.683	3	.097	1	1.169e-2	1	NC	15	NC	3
54			min	0	15	-1.076	1	.004	15	-7.478e-3	3	337.641	1	2717.888	1
55		9	max	0	1	.545	3	.03	1	1.267e-2	1	NC	15	NC	2
56			min	0	15	-.94	1	-.004	10	-8.132e-3	3	410.663	1	9318.93	1
57		10	max	0	1	.481	3	.017	3	1.366e-2	1	NC	5	NC	1
58			min	0	1	-.876	1	-.011	2	-8.786e-3	3	457.464	1	NC	1
59		11	max	0	15	.545	3	.03	1	1.267e-2	1	NC	15	NC	2
60			min	0	1	-.94	1	-.004	10	-8.132e-3	3	410.663	1	9318.93	1
61		12	max	0	15	.683	3	.097	1	1.169e-2	1	NC	15	NC	3
62			min	0	1	-1.076	1	.004	15	-7.478e-3	3	337.641	1	2717.888	1
63		13	max	0	15	.827	3	.163	1	1.07e-2	1	NC	15	NC	3
64			min	0	1	-1.212	1	.007	15	-6.824e-3	3	286.454	1	1604.947	1
65		14	max	0	15	.928	3	.203	1	9.715e-3	1	9145.846	15	NC	3
66			min	0	1	-1.296	1	.009	15	-6.17e-3	3	262.004	1	1284.38	1
67		15	max	0	15	.95	3	.205	1	8.729e-3	1	9139.766	15	NC	3
68			min	0	1	-1.293	1	.009	15	-5.516e-3	3	263.032	1	1271.431	1
69		16	max	0	15	.877	3	.169	1	7.742e-3	1	NC	15	NC	3
70			min	0	1	-1.183	1	.007	15	-4.862e-3	3	296.049	1	1550.151	1
71		17	max	0	15	.707	3	.105	1	6.756e-3	1	NC	15	NC	3
72			min	0	1	-.968	1	.004	15	-4.208e-3	3	392.898	1	2507.754	1
73		18	max	0	15	.458	3	.038	1	5.77e-3	1	NC	5	NC	2
74			min	0	1	-.666	1	.002	15	-3.554e-3	3	728.223	1	7239.7	1
75		19	max	0	15	.16	3	.005	3	4.783e-3	1	NC	1	NC	1
76			min	0	1	-.312	1	-.002	2	-2.9e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.164	3	.005	3	2.437e-3	3	NC	1	NC	1
78			min	0	1	-.311	1	-.002	2	-4.904e-3	1	NC	1	NC	1
79		2	max	0	15	.342	3	.038	1	2.992e-3	3	NC	5	NC	2
80			min	0	1	-.705	1	.002	15	-5.922e-3	1	654.159	1	7206.709	1
81		3	max	0	15	.494	3	.105	1	3.547e-3	3	NC	15	NC	3
82			min	0	1	-1.04	1	.004	15	-6.94e-3	1	353.807	1	2501.114	1
83		4	max	0	15	.605	3	.169	1	4.102e-3	3	NC	15	NC	3
84			min	0	1	-1.275	1	.007	15	-7.958e-3	1	267.734	1	1547	1
85		5	max	0	15	.665	3	.205	1	4.657e-3	3	9151.848	15	NC	3
86			min	0	1	-1.388	1	.009	15	-8.976e-3	1	239.46	1	1269.118	1
87		6	max	0	15	.675	3	.203	1	5.211e-3	3	9160.374	15	NC	3
88			min	0	1	-1.382	1	.009	15	-9.994e-3	1	240.946	1	1281.988	1
89		7	max	0	15	.641	3	.163	1	5.766e-3	3	NC	15	NC	3
90			min	0	1	-1.276	1	.007	15	-1.101e-2	1	267.508	1	1601.333	1
91		8	max	0	15	.582	3	.097	1	6.321e-3	3	NC	15	NC	3
92			min	0	1	-1.111	1	.004	15	-1.203e-2	1	322.62	1	2708.102	1
93		9	max	0	15	.521	3	.03	1	6.876e-3	3	NC	15	NC	2
94			min	0	1	-.949	1	-.003	10	-1.305e-2	1	404.212	1	9210.995	1
95		10	max	0	1	.492	3	.016	3	7.43e-3	3	NC	5	NC	1
96			min	0	1	-.874	1	-.011	2	-1.407e-2	1	458.698	1	NC	1
97		11	max	0	1	.521	3	.03	1	6.876e-3	3	NC	15	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-.949	1	-.003	10	-1.305e-2	1	404.212	1	9210.995	1
99		12	max	0	1	.582	3	.097	1	6.321e-3	3	NC	15	NC	3
100			min	0	15	-1.111	1	.004	15	-1.203e-2	1	322.62	1	2708.102	1
101		13	max	0	1	.641	3	.163	1	5.766e-3	3	NC	15	NC	3
102			min	0	15	-1.276	1	.007	15	-1.101e-2	1	267.508	1	1601.333	1
103		14	max	0	1	.675	3	.203	1	5.211e-3	3	9160.374	15	NC	3
104			min	0	15	-1.382	1	.009	15	-9.994e-3	1	240.946	1	1281.988	1
105		15	max	0	1	.665	3	.205	1	4.657e-3	3	9151.848	15	NC	3
106			min	0	15	-1.388	1	.009	15	-8.976e-3	1	239.46	1	1269.118	1
107		16	max	0	1	.605	3	.169	1	4.102e-3	3	NC	15	NC	3
108			min	0	15	-1.275	1	.007	15	-7.958e-3	1	267.734	1	1547	1
109		17	max	0	1	.494	3	.105	1	3.547e-3	3	NC	15	NC	3
110			min	0	15	-1.04	1	.004	15	-6.94e-3	1	353.807	1	2501.114	1
111		18	max	0	1	.342	3	.038	1	2.992e-3	3	NC	5	NC	2
112			min	0	15	-.705	1	.002	15	-5.922e-3	1	654.159	1	7206.709	1
113		19	max	0	1	.164	3	.005	3	2.437e-3	3	NC	1	NC	1
114			min	0	15	-.311	1	-.002	2	-4.904e-3	1	NC	1	NC	1
115	M16	1	max	0	15	.092	1	.004	3	4.292e-3	3	NC	1	NC	1
116			min	-.001	1	-.054	3	-.002	2	-7.113e-3	1	NC	1	NC	1
117		2	max	0	15	.054	3	.053	1	5.114e-3	3	NC	5	NC	2
118			min	-.001	1	-.204	2	.002	15	-8.199e-3	1	902.284	2	5036.464	1
119		3	max	0	15	.138	3	.129	1	5.935e-3	3	NC	5	NC	3
120			min	-.001	1	-.433	2	.005	15	-9.286e-3	1	501.553	2	2034.669	1
121		4	max	0	15	.184	3	.195	1	6.757e-3	3	NC	5	NC	3
122			min	0	1	-.565	2	.008	15	-1.037e-2	1	398.812	2	1337.635	1
123		5	max	0	15	.185	3	.229	1	7.579e-3	3	NC	5	NC	3
124			min	0	1	-.584	2	.01	15	-1.146e-2	1	387.513	2	1133.957	1
125		6	max	0	15	.142	3	.223	1	8.4e-3	3	NC	5	NC	3
126			min	0	1	-.492	2	.009	15	-1.254e-2	1	449.454	2	1168.901	1
127		7	max	0	15	.064	3	.176	1	9.222e-3	3	NC	5	NC	3
128			min	0	1	-.314	2	.007	15	-1.363e-2	1	652.44	2	1479.57	1
129		8	max	0	15	0	15	.104	1	1.004e-2	3	NC	3	NC	3
130			min	0	1	-.093	2	.005	15	-1.472e-2	1	1478.119	2	2521.83	1
131		9	max	0	15	.135	1	.032	1	1.087e-2	3	NC	2	NC	2
132			min	0	1	-.111	3	-.003	10	-1.58e-2	1	4487.326	3	8528.307	1
133		10	max	0	1	.223	1	.014	3	1.169e-2	3	NC	5	NC	1
134			min	0	1	-.148	3	-.01	2	-1.689e-2	1	1969.785	1	NC	1
135		11	max	0	1	.135	1	.032	1	1.087e-2	3	NC	2	NC	2
136			min	0	15	-.111	3	-.003	10	-1.58e-2	1	4487.326	3	8528.307	1
137		12	max	0	1	0	15	.104	1	1.004e-2	3	NC	3	NC	3
138			min	0	15	-.093	2	.005	15	-1.472e-2	1	1478.119	2	2521.83	1
139		13	max	0	1	.064	3	.176	1	9.222e-3	3	NC	5	NC	3
140			min	0	15	-.314	2	.007	15	-1.363e-2	1	652.44	2	1479.57	1
141		14	max	0	1	.142	3	.223	1	8.4e-3	3	NC	5	NC	3
142			min	0	15	-.492	2	.009	15	-1.254e-2	1	449.454	2	1168.901	1
143		15	max	0	1	.185	3	.229	1	7.579e-3	3	NC	5	NC	3
144			min	0	15	-.584	2	.01	15	-1.146e-2	1	387.513	2	1133.957	1
145		16	max	0	1	.184	3	.195	1	6.757e-3	3	NC	5	NC	3
146			min	0	15	-.565	2	.008	15	-1.037e-2	1	398.812	2	1337.635	1
147		17	max	.001	1	.138	3	.129	1	5.935e-3	3	NC	5	NC	3
148			min	0	15	-.433	2	.005	15	-9.286e-3	1	501.553	2	2034.669	1
149		18	max	.001	1	.054	3	.053	1	5.114e-3	3	NC	5	NC	2
150			min	0	15	-.204	2	.002	15	-8.199e-3	1	902.284	2	5036.464	1
151		19	max	.001	1	.092	1	.004	3	4.292e-3	3	NC	1	NC	1
152			min	0	15	-.054	3	-.002	2	-7.113e-3	1	NC	1	NC	1
153	M2	1	max	.006	1	.005	2	.01	1	-1.124e-5	15	NC	1	NC	2
154			min	-.006	3	-.009	3	0	15	-2.725e-4	1	NC	1	6428.606	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	1	.004	2	.009	1	-1.054e-5	15	NC	1	NC	2
156		min	-.006	3	-.008	3	0	15	-2.556e-4	1	NC	1	7012.91	1
157	3	max	.005	1	.003	2	.008	1	-9.844e-6	15	NC	1	NC	2
158		min	-.005	3	-.008	3	0	15	-2.387e-4	1	NC	1	7709.575	1
159	4	max	.005	1	.002	2	.007	1	-9.147e-6	15	NC	1	NC	2
160		min	-.005	3	-.008	3	0	15	-2.218e-4	1	NC	1	8548.479	1
161	5	max	.005	1	.002	2	.007	1	-8.449e-6	15	NC	1	NC	2
162		min	-.005	3	-.008	3	0	15	-2.048e-4	1	NC	1	9570.434	1
163	6	max	.004	1	0	2	.006	1	-7.752e-6	15	NC	1	NC	1
164		min	-.004	3	-.007	3	0	15	-1.879e-4	1	NC	1	NC	1
165	7	max	.004	1	0	2	.005	1	-7.054e-6	15	NC	1	NC	1
166		min	-.004	3	-.007	3	0	15	-1.71e-4	1	NC	1	NC	1
167	8	max	.004	1	0	2	.004	1	-6.357e-6	15	NC	1	NC	1
168		min	-.004	3	-.007	3	0	15	-1.541e-4	1	NC	1	NC	1
169	9	max	.003	1	0	2	.004	1	-5.659e-6	15	NC	1	NC	1
170		min	-.003	3	-.006	3	0	15	-1.372e-4	1	NC	1	NC	1
171	10	max	.003	1	0	15	.003	1	-4.962e-6	15	NC	1	NC	1
172		min	-.003	3	-.006	3	0	15	-1.202e-4	1	NC	1	NC	1
173	11	max	.003	1	0	15	.002	1	-4.265e-6	15	NC	1	NC	1
174		min	-.003	3	-.005	3	0	15	-1.033e-4	1	NC	1	NC	1
175	12	max	.002	1	0	15	.002	1	-3.567e-6	15	NC	1	NC	1
176		min	-.002	3	-.005	3	0	15	-8.64e-5	1	NC	1	NC	1
177	13	max	.002	1	0	15	.001	1	-2.87e-6	15	NC	1	NC	1
178		min	-.002	3	-.004	3	0	15	-6.948e-5	1	NC	1	NC	1
179	14	max	.002	1	0	15	.001	1	-2.172e-6	15	NC	1	NC	1
180		min	-.002	3	-.004	3	0	15	-5.256e-5	1	NC	1	NC	1
181	15	max	.001	1	0	15	0	1	-1.475e-6	15	NC	1	NC	1
182		min	-.001	3	-.003	3	0	15	-3.564e-5	1	NC	1	NC	1
183	16	max	0	1	0	15	0	1	-7.776e-7	15	NC	1	NC	1
184		min	0	3	-.003	4	0	15	-1.872e-5	1	NC	1	NC	1
185	17	max	0	1	0	15	0	1	-8.013e-8	15	NC	1	NC	1
186		min	0	3	-.002	4	0	15	-1.799e-6	1	NC	1	NC	1
187	18	max	0	1	0	15	0	1	1.512e-5	1	NC	1	NC	1
188		min	0	3	-.001	4	0	15	5.469e-7	12	NC	1	NC	1
189	19	max	0	1	0	1	0	1	3.204e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	1.315e-6	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	-4.327e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-1.054e-5	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	1.771e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	7.303e-7	15	NC	1	NC	1
195	3	max	0	3	0	15	0	1	4.595e-5	1	NC	1	NC	1
196		min	0	2	-.004	4	0	15	1.893e-6	15	NC	1	NC	1
197	4	max	0	3	-.001	15	0	1	7.419e-5	1	NC	1	NC	1
198		min	0	2	-.006	4	0	15	3.056e-6	15	NC	1	NC	1
199	5	max	.001	3	-.002	15	0	1	1.024e-4	1	NC	1	NC	1
200		min	0	2	-.007	4	0	15	4.219e-6	15	NC	1	NC	1
201	6	max	.001	3	-.002	15	.001	1	1.307e-4	1	NC	1	NC	1
202		min	0	2	-.009	4	0	15	5.382e-6	15	NC	1	NC	1
203	7	max	.002	3	-.003	15	.001	1	1.589e-4	1	NC	1	NC	1
204		min	-.001	2	-.011	4	0	15	6.545e-6	15	8602.873	4	NC	1
205	8	max	.002	3	-.003	15	.002	1	1.872e-4	1	NC	1	NC	1
206		min	-.001	2	-.012	4	0	15	7.708e-6	15	7723.893	4	NC	1
207	9	max	.002	3	-.003	15	.002	1	2.154e-4	1	NC	2	NC	1
208		min	-.002	2	-.013	4	0	15	8.872e-6	15	7204.213	4	NC	1
209	10	max	.002	3	-.003	15	.003	1	2.437e-4	1	NC	3	NC	1
210		min	-.002	2	-.013	4	0	15	1.003e-5	15	6953.307	4	NC	1
211	11	max	.003	3	-.003	15	.003	1	2.719e-4	1	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.002	2	-.014	4	0	15	1.12e-5	15	6934.212	4	NC	1
213		max	.003	3	-.003	15	.004	1	3.001e-4	1	NC	3	NC	1
214		min	-.002	2	-.013	4	0	15	1.236e-5	15	7149.359	4	NC	1
215		max	.003	3	-.003	15	.004	1	3.284e-4	1	NC	1	NC	1
216		min	-.002	2	-.012	4	0	15	1.352e-5	15	7643.317	4	NC	1
217		max	.004	3	-.003	15	.005	1	3.566e-4	1	NC	1	NC	1
218		min	-.003	2	-.011	4	0	15	1.469e-5	15	8525.899	4	NC	1
219		max	.004	3	-.002	15	.006	1	3.849e-4	1	NC	1	NC	1
220		min	-.003	2	-.01	4	0	15	1.585e-5	15	NC	1	NC	1
221		max	.004	3	-.002	15	.006	1	4.131e-4	1	NC	1	NC	1
222		min	-.003	2	-.008	4	0	15	1.701e-5	15	NC	1	NC	1
223		max	.004	3	-.001	15	.007	1	4.414e-4	1	NC	1	NC	1
224		min	-.003	2	-.006	1	0	15	1.818e-5	15	NC	1	NC	1
225		max	.005	3	0	15	.008	1	4.696e-4	1	NC	1	NC	1
226		min	-.003	2	-.005	1	0	15	1.934e-5	15	NC	1	NC	1
227		max	.005	3	0	15	.009	1	4.978e-4	1	NC	1	NC	2
228		min	-.004	2	-.003	1	0	15	2.05e-5	15	NC	1	9754.599	1
229	M4	max	.003	1	.003	2	0	15	6.626e-5	1	NC	1	NC	3
230		min	0	3	-.005	3	-.009	1	2.74e-6	15	NC	1	2673.069	1
231		max	.003	1	.003	2	0	15	6.626e-5	1	NC	1	NC	3
232		min	0	3	-.005	3	-.009	1	2.74e-6	15	NC	1	2906.488	1
233		max	.002	1	.003	2	0	15	6.626e-5	1	NC	1	NC	3
234		min	0	3	-.004	3	-.008	1	2.74e-6	15	NC	1	3184.309	1
235		max	.002	1	.003	2	0	15	6.626e-5	1	NC	1	NC	3
236		min	0	3	-.004	3	-.007	1	2.74e-6	15	NC	1	3518.064	1
237		max	.002	1	.002	2	0	15	6.626e-5	1	NC	1	NC	3
238		min	0	3	-.004	3	-.006	1	2.74e-6	15	NC	1	3923.424	1
239		max	.002	1	.002	2	0	15	6.626e-5	1	NC	1	NC	2
240		min	0	3	-.004	3	-.006	1	2.74e-6	15	NC	1	4422.123	1
241		max	.002	1	.002	2	0	15	6.626e-5	1	NC	1	NC	2
242		min	0	3	-.003	3	-.005	1	2.74e-6	15	NC	1	5045.01	1
243		max	.002	1	.002	2	0	15	6.626e-5	1	NC	1	NC	2
244		min	0	3	-.003	3	-.004	1	2.74e-6	15	NC	1	5837.077	1
245		max	.002	1	.002	2	0	15	6.626e-5	1	NC	1	NC	2
246		min	0	3	-.003	3	-.004	1	2.74e-6	15	NC	1	6866.057	1
247		max	.001	1	.002	2	0	15	6.626e-5	1	NC	1	NC	2
248		min	0	3	-.002	3	-.003	1	2.74e-6	15	NC	1	8237.831	1
249		max	.001	1	.001	2	0	15	6.626e-5	1	NC	1	NC	1
250		min	0	3	-.002	3	-.002	1	2.74e-6	15	NC	1	NC	1
251		max	.001	1	.001	2	0	15	6.626e-5	1	NC	1	NC	1
252		min	0	3	-.002	3	-.002	1	2.74e-6	15	NC	1	NC	1
253		max	0	1	.001	2	0	15	6.626e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	-.001	1	2.74e-6	15	NC	1	NC	1
255		max	0	1	0	2	0	15	6.626e-5	1	NC	1	NC	1
256		min	0	3	-.001	3	-.001	1	2.74e-6	15	NC	1	NC	1
257		max	0	1	0	2	0	15	6.626e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	2.74e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	6.626e-5	1	NC	1	NC	1
260		min	0	3	0	3	0	1	2.74e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.626e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	2.74e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.626e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.74e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.626e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.74e-6	15	NC	1	NC	1
267	M6	max	.019	1	.019	2	0	1	0	1	NC	3	NC	1
268		min	-.02	3	-.027	3	0	1	0	1	3270.598	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.018	1	.017	2	0	1	0	1	NC	3	NC	1
270		min	-.019	3	-.026	3	0	1	0	1	3596.194	2	NC	1
271	3	max	.017	1	.016	2	0	1	0	1	NC	3	NC	1
272		min	-.018	3	-.024	3	0	1	0	1	3990.343	2	NC	1
273	4	max	.016	1	.014	2	0	1	0	1	NC	3	NC	1
274		min	-.016	3	-.023	3	0	1	0	1	4472.946	2	NC	1
275	5	max	.015	1	.012	2	0	1	0	1	NC	1	NC	1
276		min	-.015	3	-.021	3	0	1	0	1	5071.941	2	NC	1
277	6	max	.014	1	.011	2	0	1	0	1	NC	1	NC	1
278		min	-.014	3	-.02	3	0	1	0	1	5827.546	2	NC	1
279	7	max	.013	1	.009	2	0	1	0	1	NC	1	NC	1
280		min	-.013	3	-.018	3	0	1	0	1	6799.393	2	NC	1
281	8	max	.012	1	.008	2	0	1	0	1	NC	1	NC	1
282		min	-.012	3	-.017	3	0	1	0	1	8079.05	2	NC	1
283	9	max	.011	1	.006	2	0	1	0	1	NC	1	NC	1
284		min	-.011	3	-.015	3	0	1	0	1	9813.189	2	NC	1
285	10	max	.01	1	.005	2	0	1	0	1	NC	1	NC	1
286		min	-.01	3	-.014	3	0	1	0	1	NC	1	NC	1
287	11	max	.009	1	.004	2	0	1	0	1	NC	1	NC	1
288		min	-.009	3	-.012	3	0	1	0	1	NC	1	NC	1
289	12	max	.007	1	.003	2	0	1	0	1	NC	1	NC	1
290		min	-.008	3	-.011	3	0	1	0	1	NC	1	NC	1
291	13	max	.006	1	.002	2	0	1	0	1	NC	1	NC	1
292		min	-.007	3	-.009	3	0	1	0	1	NC	1	NC	1
293	14	max	.005	1	.001	2	0	1	0	1	NC	1	NC	1
294		min	-.005	3	-.008	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	1	0	2	0	1	0	1	NC	1	NC	1
296		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	15	0	1	0	1	NC	1	NC	1
308		min	0	2	-.002	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	0	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.004	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
313	5	max	.003	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.003	2	-.008	3	0	1	0	1	NC	1	NC	1
315	6	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.004	2	-.01	3	0	1	0	1	NC	1	NC	1
317	7	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.005	2	-.011	3	0	1	0	1	8822.976	4	NC	1
319	8	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.006	2	-.012	3	0	1	0	1	7906.975	4	NC	1
321	9	max	.007	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.007	2	-.013	4	0	1	0	1	7363.744	4	NC	1
323	10	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.007	2	-.014	4	0	1	0	1	7098.218	4	NC	1
325	11	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.008	2	-.014	4	0	1	0	1	7071.131	4	NC	1
327		12	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.009	2	-.013	4	0	1	0	1	7283.938	4	NC	1
329		13	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.01	2	-.013	4	0	1	0	1	7781.289	4	NC	1
331		14	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.011	2	-.012	4	0	1	0	1	8674.332	4	NC	1
333		15	max	.012	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.011	2	-.01	3	0	1	0	1	NC	1	NC	1
335		16	max	.013	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.012	2	-.009	1	0	1	0	1	NC	1	NC	1
337		17	max	.014	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.013	2	-.008	1	0	1	0	1	NC	1	NC	1
339		18	max	.014	3	0	15	0	1	0	1	NC	1	NC	1
340			min	-.014	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.015	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.015	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.014	2	0	1	0	1	NC	1	NC	1
344			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.013	2	0	1	0	1	NC	1	NC	1
346			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
347		3	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.012	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.009	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.003	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.002	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	1	.005	2	0	15	2.725e-4	1	NC	1	NC	2
382			min	-.006	3	-.009	3	-.01	1	1.124e-5	15	NC	1	6428.606	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.006	1	.004	2	0	15	2.556e-4	1	NC	1	NC	2
384		min	-.006	3	-.008	3	-.009	1	1.054e-5	15	NC	1	7012.91	1
385	3	max	.005	1	.003	2	0	15	2.387e-4	1	NC	1	NC	2
386		min	-.005	3	-.008	3	-.008	1	9.844e-6	15	NC	1	7709.575	1
387	4	max	.005	1	.002	2	0	15	2.218e-4	1	NC	1	NC	2
388		min	-.005	3	-.008	3	-.007	1	9.147e-6	15	NC	1	8548.479	1
389	5	max	.005	1	.002	2	0	15	2.048e-4	1	NC	1	NC	2
390		min	-.005	3	-.008	3	-.007	1	8.449e-6	15	NC	1	9570.434	1
391	6	max	.004	1	0	2	0	15	1.879e-4	1	NC	1	NC	1
392		min	-.004	3	-.007	3	-.006	1	7.752e-6	15	NC	1	NC	1
393	7	max	.004	1	0	2	0	15	1.71e-4	1	NC	1	NC	1
394		min	-.004	3	-.007	3	-.005	1	7.054e-6	15	NC	1	NC	1
395	8	max	.004	1	0	2	0	15	1.541e-4	1	NC	1	NC	1
396		min	-.004	3	-.007	3	-.004	1	6.357e-6	15	NC	1	NC	1
397	9	max	.003	1	0	2	0	15	1.372e-4	1	NC	1	NC	1
398		min	-.003	3	-.006	3	-.004	1	5.659e-6	15	NC	1	NC	1
399	10	max	.003	1	0	15	0	15	1.202e-4	1	NC	1	NC	1
400		min	-.003	3	-.006	3	-.003	1	4.962e-6	15	NC	1	NC	1
401	11	max	.003	1	0	15	0	15	1.033e-4	1	NC	1	NC	1
402		min	-.003	3	-.005	3	-.002	1	4.265e-6	15	NC	1	NC	1
403	12	max	.002	1	0	15	0	15	8.64e-5	1	NC	1	NC	1
404		min	-.002	3	-.005	3	-.002	1	3.567e-6	15	NC	1	NC	1
405	13	max	.002	1	0	15	0	15	6.948e-5	1	NC	1	NC	1
406		min	-.002	3	-.004	3	-.001	1	2.87e-6	15	NC	1	NC	1
407	14	max	.002	1	0	15	0	15	5.256e-5	1	NC	1	NC	1
408		min	-.002	3	-.004	3	-.001	1	2.172e-6	15	NC	1	NC	1
409	15	max	.001	1	0	15	0	15	3.564e-5	1	NC	1	NC	1
410		min	-.001	3	-.003	3	0	1	1.475e-6	15	NC	1	NC	1
411	16	max	0	1	0	15	0	15	1.872e-5	1	NC	1	NC	1
412		min	0	3	-.003	4	0	1	7.776e-7	15	NC	1	NC	1
413	17	max	0	1	0	15	0	15	1.799e-6	1	NC	1	NC	1
414		min	0	3	-.002	4	0	1	8.013e-8	15	NC	1	NC	1
415	18	max	0	1	0	15	0	15	-5.469e-7	12	NC	1	NC	1
416		min	0	3	-.001	4	0	1	-1.512e-5	1	NC	1	NC	1
417	19	max	0	1	0	1	0	1	-1.315e-6	15	NC	1	NC	1
418		min	0	1	0	1	0	1	-3.204e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	1.054e-5	1	NC	1	NC	1
420		min	0	1	0	1	0	1	4.327e-7	15	NC	1	NC	1
421	2	max	0	3	0	15	0	15	-7.303e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	1	-1.771e-5	1	NC	1	NC	1
423	3	max	0	3	0	15	0	15	-1.893e-6	15	NC	1	NC	1
424		min	0	2	-.004	4	0	1	-4.595e-5	1	NC	1	NC	1
425	4	max	0	3	-.001	15	0	15	-3.056e-6	15	NC	1	NC	1
426		min	0	2	-.006	4	0	1	-7.419e-5	1	NC	1	NC	1
427	5	max	.001	3	-.002	15	0	15	-4.219e-6	15	NC	1	NC	1
428		min	0	2	-.007	4	0	1	-1.024e-4	1	NC	1	NC	1
429	6	max	.001	3	-.002	15	0	15	-5.382e-6	15	NC	1	NC	1
430		min	0	2	-.009	4	-.001	1	-1.307e-4	1	NC	1	NC	1
431	7	max	.002	3	-.003	15	0	15	-6.545e-6	15	NC	1	NC	1
432		min	-.001	2	-.011	4	-.001	1	-1.589e-4	1	8602.873	4	NC	1
433	8	max	.002	3	-.003	15	0	15	-7.708e-6	15	NC	1	NC	1
434		min	-.001	2	-.012	4	-.002	1	-1.872e-4	1	7723.893	4	NC	1
435	9	max	.002	3	-.003	15	0	15	-8.872e-6	15	NC	2	NC	1
436		min	-.002	2	-.013	4	-.002	1	-2.154e-4	1	7204.213	4	NC	1
437	10	max	.002	3	-.003	15	0	15	-1.003e-5	15	NC	3	NC	1
438		min	-.002	2	-.013	4	-.003	1	-2.437e-4	1	6953.307	4	NC	1
439	11	max	.003	3	-.003	15	0	15	-1.12e-5	15	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.002	2	-.014	4	-.003	1	-2.719e-4	1	6934.212	4	NC	1
441		max	.003	3	-.003	15	0	15	-1.236e-5	15	NC	3	NC	1
442		min	-.002	2	-.013	4	-.004	1	-3.001e-4	1	7149.359	4	NC	1
443		max	.003	3	-.003	15	0	15	-1.352e-5	15	NC	1	NC	1
444		min	-.002	2	-.012	4	-.004	1	-3.284e-4	1	7643.317	4	NC	1
445		max	.004	3	-.003	15	0	15	-1.469e-5	15	NC	1	NC	1
446		min	-.003	2	-.011	4	-.005	1	-3.566e-4	1	8525.899	4	NC	1
447		max	.004	3	-.002	15	0	15	-1.585e-5	15	NC	1	NC	1
448		min	-.003	2	-.01	4	-.006	1	-3.849e-4	1	NC	1	NC	1
449		max	.004	3	-.002	15	0	15	-1.701e-5	15	NC	1	NC	1
450		min	-.003	2	-.008	4	-.006	1	-4.131e-4	1	NC	1	NC	1
451		max	.004	3	-.001	15	0	15	-1.818e-5	15	NC	1	NC	1
452		min	-.003	2	-.006	1	-.007	1	-4.414e-4	1	NC	1	NC	1
453		max	.005	3	0	15	0	15	-1.934e-5	15	NC	1	NC	1
454		min	-.003	2	-.005	1	-.008	1	-4.696e-4	1	NC	1	NC	1
455		max	.005	3	0	15	0	15	-2.05e-5	15	NC	1	NC	2
456		min	-.004	2	-.003	1	-.009	1	-4.978e-4	1	NC	1	9754.599	1
457	M12	max	.003	1	.003	2	.009	1	-2.74e-6	15	NC	1	NC	3
458		min	0	3	-.005	3	0	15	-6.626e-5	1	NC	1	2673.069	1
459		max	.003	1	.003	2	.009	1	-2.74e-6	15	NC	1	NC	3
460		min	0	3	-.005	3	0	15	-6.626e-5	1	NC	1	2906.488	1
461		max	.002	1	.003	2	.008	1	-2.74e-6	15	NC	1	NC	3
462		min	0	3	-.004	3	0	15	-6.626e-5	1	NC	1	3184.309	1
463		max	.002	1	.003	2	.007	1	-2.74e-6	15	NC	1	NC	3
464		min	0	3	-.004	3	0	15	-6.626e-5	1	NC	1	3518.064	1
465		max	.002	1	.002	2	.006	1	-2.74e-6	15	NC	1	NC	3
466		min	0	3	-.004	3	0	15	-6.626e-5	1	NC	1	3923.424	1
467		max	.002	1	.002	2	.006	1	-2.74e-6	15	NC	1	NC	2
468		min	0	3	-.004	3	0	15	-6.626e-5	1	NC	1	4422.123	1
469		max	.002	1	.002	2	.005	1	-2.74e-6	15	NC	1	NC	2
470		min	0	3	-.003	3	0	15	-6.626e-5	1	NC	1	5045.01	1
471		max	.002	1	.002	2	.004	1	-2.74e-6	15	NC	1	NC	2
472		min	0	3	-.003	3	0	15	-6.626e-5	1	NC	1	5837.077	1
473		max	.002	1	.002	2	.004	1	-2.74e-6	15	NC	1	NC	2
474		min	0	3	-.003	3	0	15	-6.626e-5	1	NC	1	6866.057	1
475		max	.001	1	.002	2	.003	1	-2.74e-6	15	NC	1	NC	2
476		min	0	3	-.002	3	0	15	-6.626e-5	1	NC	1	8237.831	1
477		max	.001	1	.001	2	.002	1	-2.74e-6	15	NC	1	NC	1
478		min	0	3	-.002	3	0	15	-6.626e-5	1	NC	1	NC	1
479		max	.001	1	.001	2	.002	1	-2.74e-6	15	NC	1	NC	1
480		min	0	3	-.002	3	0	15	-6.626e-5	1	NC	1	NC	1
481		max	0	1	.001	2	.001	1	-2.74e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-6.626e-5	1	NC	1	NC	1
483		max	0	1	0	2	.001	1	-2.74e-6	15	NC	1	NC	1
484		min	0	3	-.001	3	0	15	-6.626e-5	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-2.74e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-6.626e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-2.74e-6	15	NC	1	NC	1
488		min	0	3	0	3	0	15	-6.626e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.74e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-6.626e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.74e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.626e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.74e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.626e-5	1	NC	1	NC	1
495	M1	max	.006	3	.095	1	.001	1	1.73e-2	1	NC	1	NC	1
496		min	-.003	2	-.009	3	0	15	-2.353e-2	3	NC	1	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.006	3	.046	1	0	15	8.396e-3	1	NC	3	NC	1
498			min	-.003	2	-.003	3	-.007	1	-1.164e-2	3	2353.705	1	NC	1
499		3	max	.006	3	.009	3	0	15	1.504e-5	10	NC	5	NC	1
500			min	-.003	2	-.008	2	-.01	1	-2.014e-4	1	1125.547	1	NC	1
501		4	max	.006	3	.031	3	0	15	4.544e-3	1	NC	5	NC	1
502			min	-.003	2	-.068	1	-.009	1	-4.055e-3	3	702.77	1	NC	1
503		5	max	.006	3	.061	3	0	15	9.289e-3	1	NC	15	NC	1
504			min	-.002	2	-.133	1	-.006	1	-7.996e-3	3	502.558	1	NC	1
505		6	max	.006	3	.094	3	0	15	1.403e-2	1	NC	15	NC	1
506			min	-.002	2	-.196	1	-.003	1	-1.194e-2	3	393.036	1	NC	1
507		7	max	.006	3	.125	3	0	1	1.878e-2	1	9885.775	15	NC	1
508			min	-.002	2	-.253	1	0	12	-1.588e-2	3	328.761	1	NC	1
509		8	max	.006	3	.152	3	.001	1	2.352e-2	1	8777.697	15	NC	1
510			min	-.002	2	-.298	1	0	15	-1.982e-2	3	290.905	1	NC	1
511		9	max	.005	3	.169	3	0	15	2.596e-2	1	8200.43	15	NC	1
512			min	-.002	2	-.327	1	0	1	-1.976e-2	3	271.258	1	NC	1
513		10	max	.005	3	.176	3	0	1	2.687e-2	1	8024.715	15	NC	1
514			min	-.002	2	-.336	1	0	12	-1.705e-2	3	265.378	1	NC	1
515		11	max	.005	3	.172	3	0	1	2.778e-2	1	8200.192	15	NC	1
516			min	-.002	2	-.326	1	0	15	-1.434e-2	3	271.61	1	NC	1
517		12	max	.005	3	.157	3	0	15	2.627e-2	1	8777.201	15	NC	1
518			min	-.002	2	-.297	1	-.001	1	-1.177e-2	3	292.011	1	NC	1
519		13	max	.005	3	.134	3	0	15	2.115e-2	1	9884.903	15	NC	1
520			min	-.002	2	-.251	1	0	1	-9.419e-3	3	331.513	1	NC	1
521		14	max	.005	3	.104	3	.002	1	1.602e-2	1	NC	15	NC	1
522			min	-.002	2	-.193	1	0	15	-7.069e-3	3	398.983	1	NC	1
523		15	max	.005	3	.071	3	.006	1	1.089e-2	1	NC	15	NC	1
524			min	-.002	2	-.128	1	0	15	-4.718e-3	3	514.888	1	NC	1
525		16	max	.005	3	.036	3	.009	1	5.76e-3	1	NC	5	NC	1
526			min	-.002	2	-.064	1	0	15	-2.368e-3	3	728.903	1	NC	1
527		17	max	.004	3	.003	3	.009	1	6.312e-4	1	NC	5	NC	1
528			min	-.002	2	-.004	2	0	15	-1.77e-5	3	1185.025	1	NC	1
529		18	max	.004	3	.047	1	.006	1	1.035e-2	2	NC	4	NC	1
530			min	-.002	2	-.026	3	0	15	-3.841e-3	3	2505.355	1	NC	1
531		19	max	.004	3	.092	1	0	15	2.074e-2	2	NC	1	NC	1
532			min	-.002	2	-.054	3	-.001	1	-7.807e-3	3	NC	1	NC	1
533	M5	1	max	.019	3	.229	1	0	1	0	1	NC	1	NC	1
534			min	-.012	2	-.006	3	0	1	0	1	NC	1	NC	1
535		2	max	.019	3	.11	1	0	1	0	1	NC	5	NC	1
536			min	-.013	2	.001	3	0	1	0	1	961.082	1	NC	1
537		3	max	.019	3	.029	3	0	1	0	1	NC	15	NC	1
538			min	-.013	2	-.025	2	0	1	0	1	449.984	1	NC	1
539		4	max	.019	3	.091	3	0	1	0	1	9218.093	15	NC	1
540			min	-.012	2	-.189	1	0	1	0	1	273.608	1	NC	1
541		5	max	.018	3	.179	3	0	1	0	1	6455.156	15	NC	1
542			min	-.012	2	-.367	1	0	1	0	1	191.564	1	NC	1
543		6	max	.018	3	.278	3	0	1	0	1	4972.195	15	NC	1
544			min	-.012	2	-.545	1	0	1	0	1	147.497	1	NC	1
545		7	max	.018	3	.375	3	0	1	0	1	4115.307	15	NC	1
546			min	-.012	2	-.707	1	0	1	0	1	122.019	1	NC	1
547		8	max	.017	3	.456	3	0	1	0	1	3616.86	15	NC	1
548			min	-.011	2	-.836	1	0	1	0	1	107.194	1	NC	1
549		9	max	.017	3	.509	3	0	1	0	1	3361.163	15	NC	1
550			min	-.011	2	-.918	1	0	1	0	1	99.59	1	NC	1
551		10	max	.016	3	.528	3	0	1	0	1	3284.117	15	NC	1
552			min	-.011	2	-.945	1	0	1	0	1	97.327	1	NC	1
553		11	max	.016	3	.514	3	0	1	0	1	3361.244	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.011	2	-.917	1	0	1	0	1	99.731	1	NC	1
555		12	max	.016	3	.47	3	0	1	0	1	3617.055	15	NC	1
556			min	-.011	2	-.833	1	0	1	0	1	107.66	1	NC	1
557		13	max	.015	3	.399	3	0	1	0	1	4115.715	15	NC	1
558			min	-.01	2	-.7	1	0	1	0	1	123.23	1	NC	1
559		14	max	.015	3	.309	3	0	1	0	1	4973.01	15	NC	1
560			min	-.01	2	-.534	1	0	1	0	1	150.217	1	NC	1
561		15	max	.015	3	.208	3	0	1	0	1	6456.787	15	NC	1
562			min	-.01	2	-.353	1	0	1	0	1	197.448	1	NC	1
563		16	max	.014	3	.106	3	0	1	0	1	9221.538	15	NC	1
564			min	-.01	2	-.173	1	0	1	0	1	286.745	1	NC	1
565		17	max	.014	3	.009	3	0	1	0	1	NC	15	NC	1
566			min	-.01	2	-.014	2	0	1	0	1	481.802	1	NC	1
567		18	max	.014	3	.115	1	0	1	0	1	NC	5	NC	1
568			min	-.01	2	-.073	3	0	1	0	1	1046.067	1	NC	1
569		19	max	.014	3	.223	1	0	1	0	1	NC	1	NC	1
570			min	-.01	2	-.148	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.006	3	.095	1	0	15	2.353e-2	3	NC	1	NC	1
572			min	-.003	2	-.009	3	-.001	1	-1.73e-2	1	NC	1	NC	1
573		2	max	.006	3	.046	1	.007	1	1.164e-2	3	NC	3	NC	1
574			min	-.003	2	-.003	3	0	15	-8.396e-3	1	2353.705	1	NC	1
575		3	max	.006	3	.009	3	.01	1	2.014e-4	1	NC	5	NC	1
576			min	-.003	2	-.008	2	0	15	-1.504e-5	10	1125.547	1	NC	1
577		4	max	.006	3	.031	3	.009	1	4.055e-3	3	NC	5	NC	1
578			min	-.003	2	-.068	1	0	15	-4.544e-3	1	702.77	1	NC	1
579		5	max	.006	3	.061	3	.006	1	7.996e-3	3	NC	15	NC	1
580			min	-.002	2	-.133	1	0	15	-9.289e-3	1	502.558	1	NC	1
581		6	max	.006	3	.094	3	.003	1	1.194e-2	3	NC	15	NC	1
582			min	-.002	2	-.196	1	0	15	-1.403e-2	1	393.036	1	NC	1
583		7	max	.006	3	.125	3	0	12	1.588e-2	3	9885.775	15	NC	1
584			min	-.002	2	-.253	1	0	1	-1.878e-2	1	328.761	1	NC	1
585		8	max	.006	3	.152	3	0	15	1.982e-2	3	8777.697	15	NC	1
586			min	-.002	2	-.298	1	-.001	1	-2.352e-2	1	290.905	1	NC	1
587		9	max	.005	3	.169	3	0	1	1.976e-2	3	8200.43	15	NC	1
588			min	-.002	2	-.327	1	0	15	-2.596e-2	1	271.258	1	NC	1
589		10	max	.005	3	.176	3	0	12	1.705e-2	3	8024.715	15	NC	1
590			min	-.002	2	-.336	1	0	1	-2.687e-2	1	265.378	1	NC	1
591		11	max	.005	3	.172	3	0	15	1.434e-2	3	8200.192	15	NC	1
592			min	-.002	2	-.326	1	0	1	-2.778e-2	1	271.61	1	NC	1
593		12	max	.005	3	.157	3	.001	1	1.177e-2	3	8777.201	15	NC	1
594			min	-.002	2	-.297	1	0	15	-2.627e-2	1	292.011	1	NC	1
595		13	max	.005	3	.134	3	0	1	9.419e-3	3	9884.903	15	NC	1
596			min	-.002	2	-.251	1	0	15	-2.115e-2	1	331.513	1	NC	1
597		14	max	.005	3	.104	3	0	15	7.069e-3	3	NC	15	NC	1
598			min	-.002	2	-.193	1	-.002	1	-1.602e-2	1	398.983	1	NC	1
599		15	max	.005	3	.071	3	0	15	4.718e-3	3	NC	15	NC	1
600			min	-.002	2	-.128	1	-.006	1	-1.089e-2	1	514.888	1	NC	1
601		16	max	.005	3	.036	3	0	15	2.368e-3	3	NC	5	NC	1
602			min	-.002	2	-.064	1	-.009	1	-5.76e-3	1	728.903	1	NC	1
603		17	max	.004	3	.003	3	0	15	1.77e-5	3	NC	5	NC	1
604			min	-.002	2	-.004	2	-.009	1	-6.312e-4	1	1185.025	1	NC	1
605		18	max	.004	3	.047	1	0	15	3.841e-3	3	NC	4	NC	1
606			min	-.002	2	-.026	3	-.006	1	-1.035e-2	2	2505.355	1	NC	1
607		19	max	.004	3	.092	1	.001	1	7.807e-3	3	NC	1	NC	1
608			min	-.002	2	-.054	3	0	15	-2.074e-2	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1723
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
Adhesive	1723	5365	0.32	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	593	3156	0.19	Pass (Governs)	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

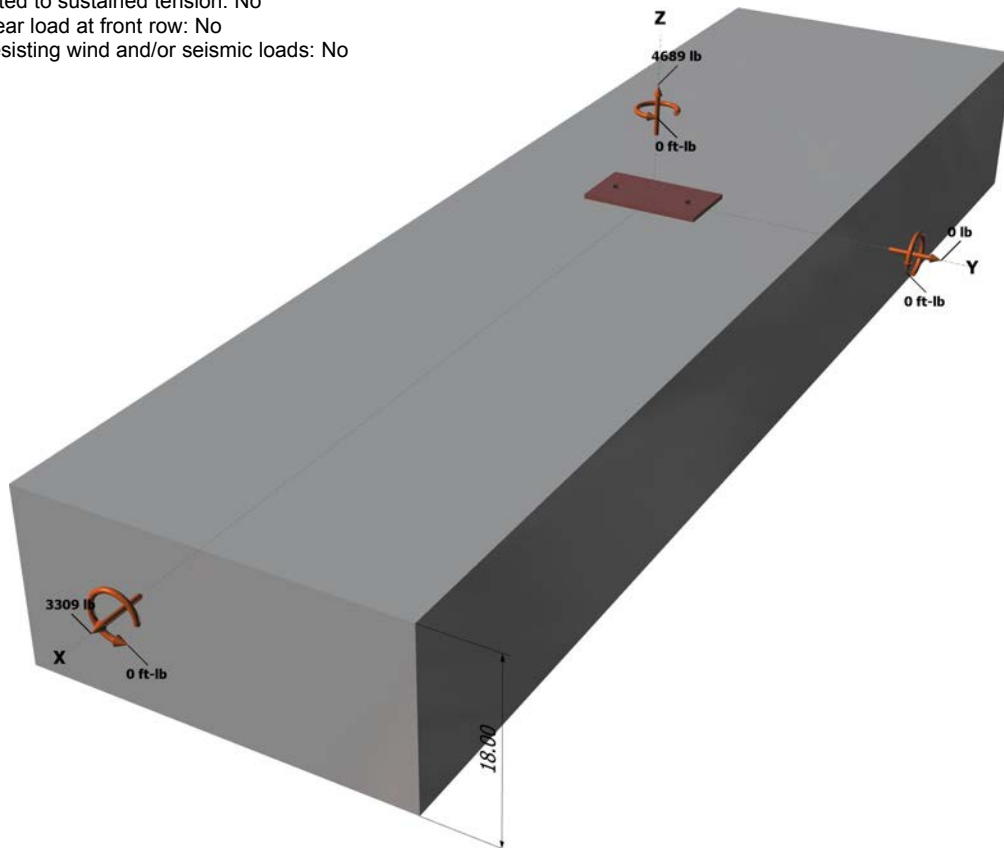
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

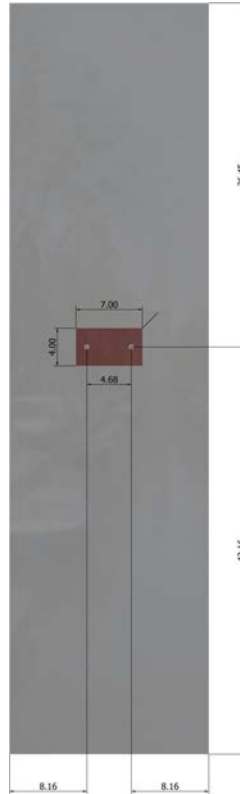
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 4689
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
Adhesive	4689	8093	0.58	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
T Concrete breakout x+	3309	5323	0.62	Pass (Governs)	
 Concrete breakout y-	1655	12241	0.14	Pass (Governs)	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
------------	------	------	---------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.