

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.05	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

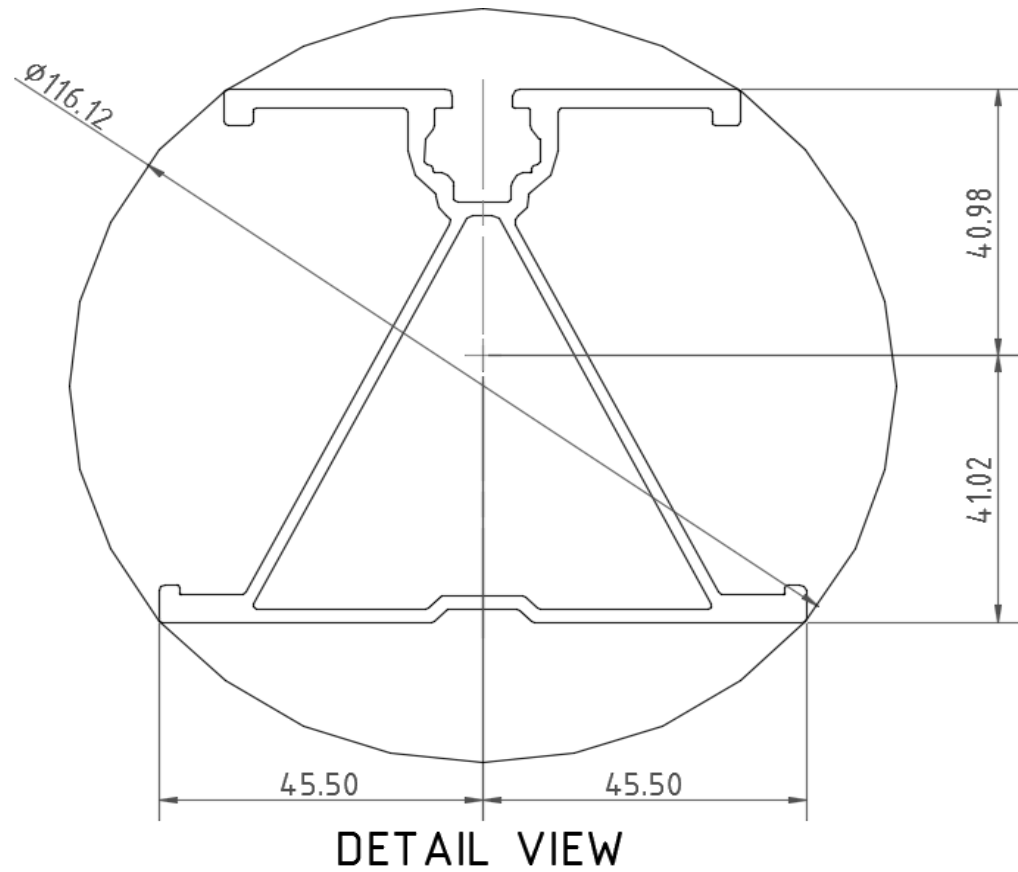
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

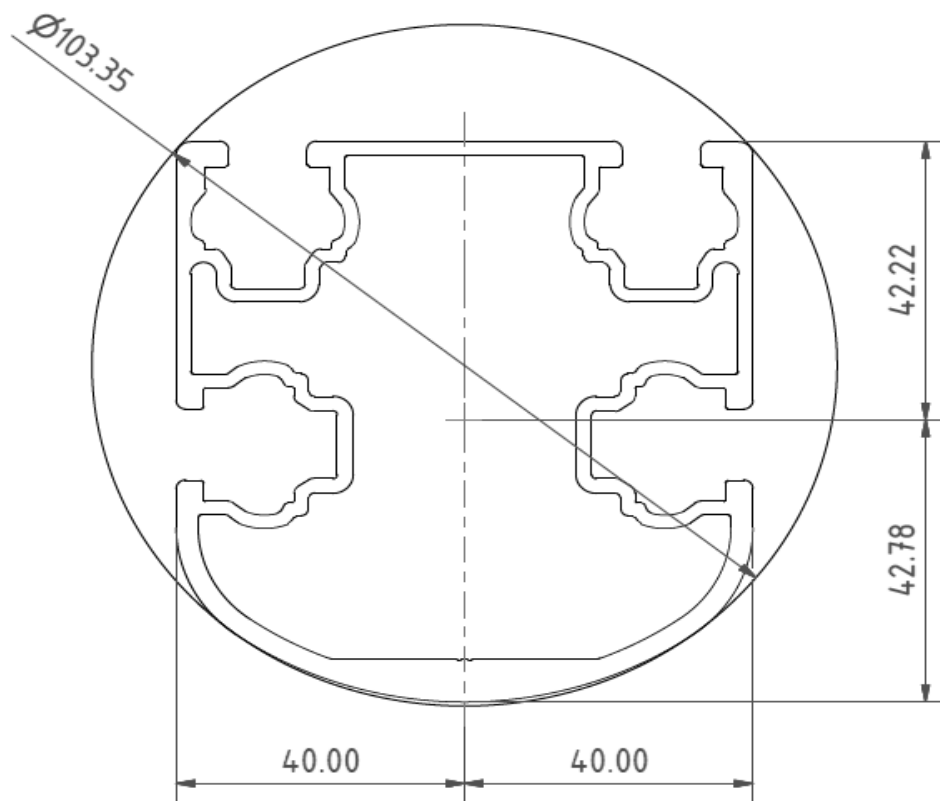
Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	117 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.121 k-ft
$M_z$ =	0.243 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>97%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

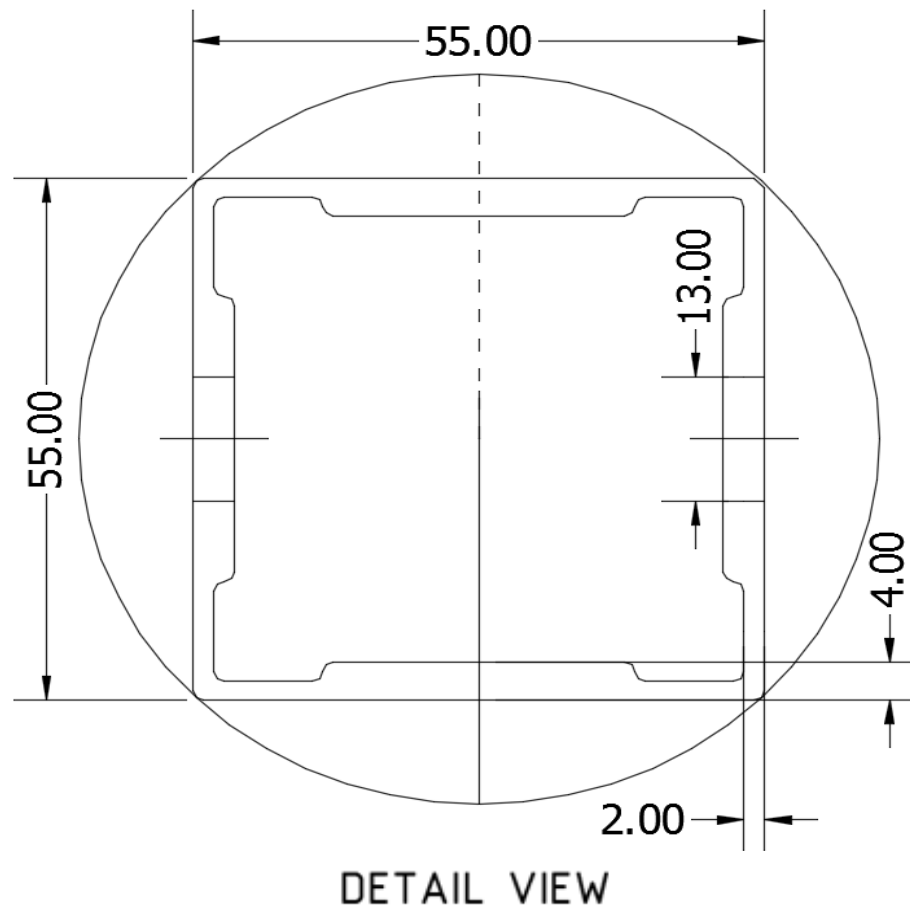
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.379 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.268 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

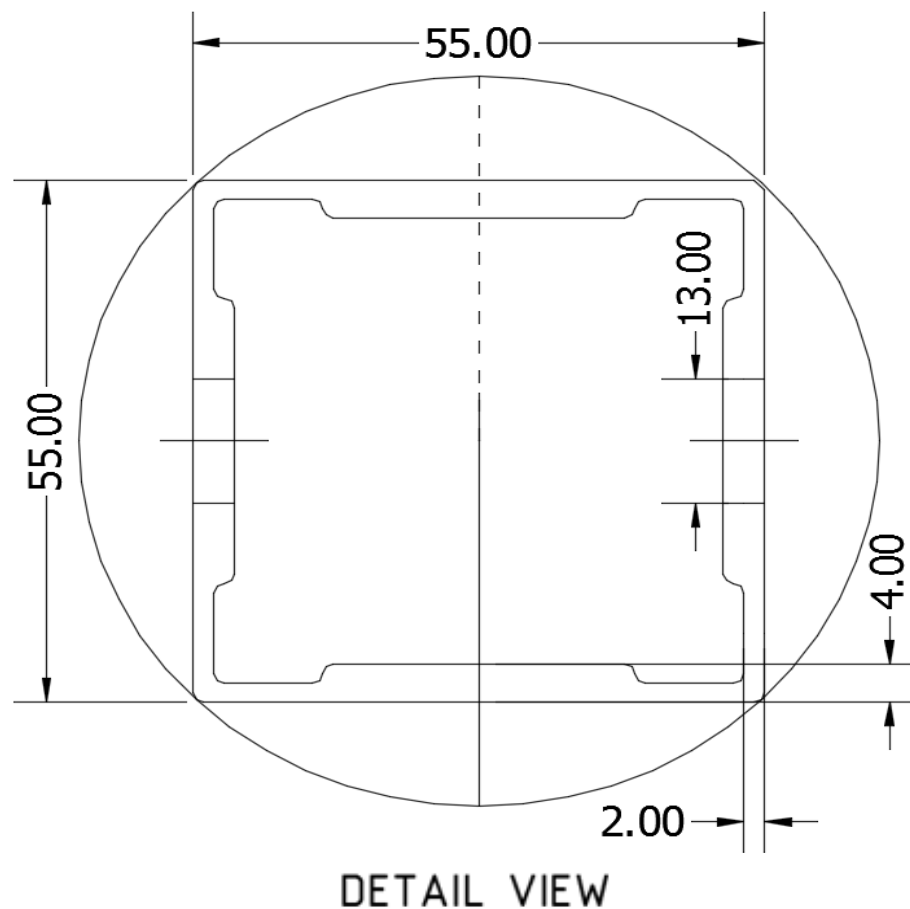
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.460 k-ft
$P_n$ =	0.671 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>35%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.395 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>20%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	48.30 in
$\Phi F_{ty \text{ AXIAL}}$ =	18.93 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	-0.316 k-ft
$P_n$ =	0.679 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	18.592 k
Utilization =	<b>27%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

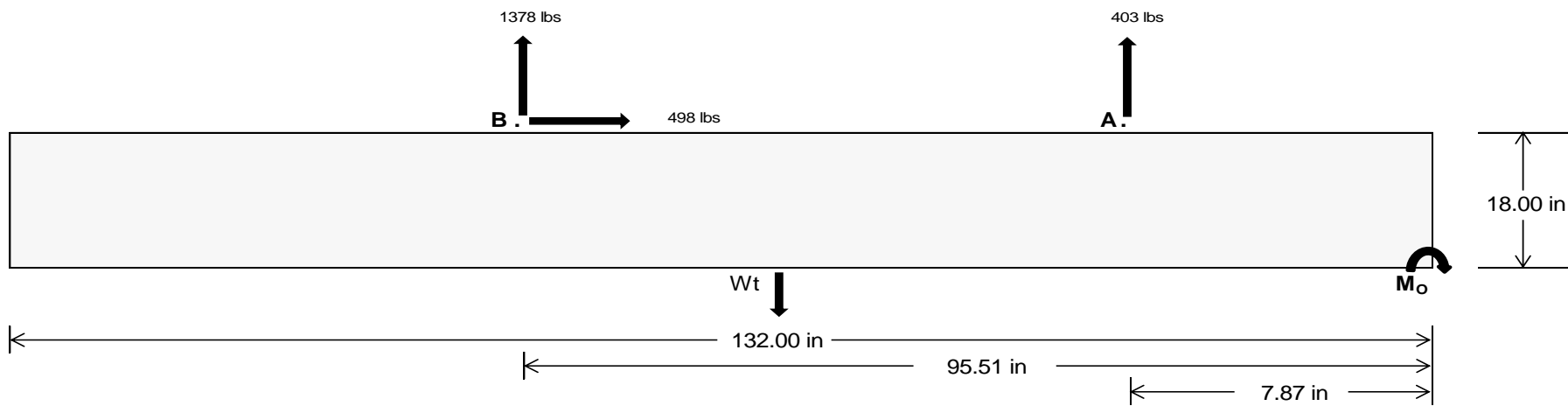
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>1689.67</b>	<b>5746.25</b> k
Compressive Load =	<b>5000.99</b>	<b>5186.92</b> k
Lateral Load =	<b>301.30</b>	<b>2070.47</b> k
Moment (Weak Axis) =	<b>0.61</b>	<b>0.41</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 143785.6$  in-lbs  
Resisting Force Required = 2178.57 lbs  
S.F. = 1.67  
Weight Required = 3630.95 lbs  
Minimum Width = 32 in in  
Weight Provided = 6380.00 lbs

### Sliding

Force = 497.59 lbs  
Friction = 0.4  
Weight Required = 1243.98 lbs  
Resisting Weight = 6380.00 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 497.59 lbs  
Cohesion = 130 psf  
Area = 29.33 ft<sup>2</sup>  
Resisting = 3190.00 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 32in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 32in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 32in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

$$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.67 \text{ ft}) = \begin{matrix} \text{Ballast Width} \\ \hline \begin{matrix} 32 \text{ in} & 33 \text{ in} & 34 \text{ in} & 35 \text{ in} \end{matrix} \\ \begin{matrix} 6380 \text{ lbs} & 6579 \text{ lbs} & 6779 \text{ lbs} & 6978 \text{ lbs} \end{matrix} \end{matrix}$$

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in
$F_A$	1681 lbs	1681 lbs	1681 lbs	1681 lbs	1825 lbs	1825 lbs	1825 lbs	1825 lbs	2504 lbs	2504 lbs	2504 lbs	2504 lbs	-806 lbs	-806 lbs	-806 lbs	-806 lbs
$F_B$	1740 lbs	1740 lbs	1740 lbs	1740 lbs	1892 lbs	1892 lbs	1892 lbs	1892 lbs	2595 lbs	2595 lbs	2595 lbs	2595 lbs	-2757 lbs	-2757 lbs	-2757 lbs	-2757 lbs
$F_V$	148 lbs	148 lbs	148 lbs	148 lbs	878 lbs	878 lbs	878 lbs	878 lbs	759 lbs	759 lbs	759 lbs	759 lbs	-995 lbs	-995 lbs	-995 lbs	-995 lbs
$P_{total}$	9801 lbs	10000 lbs	10199 lbs	10399 lbs	10098 lbs	10297 lbs	10496 lbs	10696 lbs	11479 lbs	11679 lbs	11878 lbs	12077 lbs	265 lbs	384 lbs	504 lbs	624 lbs
$M$	4082 lbs-ft	4082 lbs-ft	4082 lbs-ft	4082 lbs-ft	5504 lbs-ft	5504 lbs-ft	5504 lbs-ft	5504 lbs-ft	6885 lbs-ft	6885 lbs-ft	6885 lbs-ft	6885 lbs-ft	1381 lbs-ft	1381 lbs-ft	1381 lbs-ft	1381 lbs-ft
$e$	0.42 ft	0.41 ft	0.40 ft	0.39 ft	0.55 ft	0.53 ft	0.52 ft	0.51 ft	0.60 ft	0.59 ft	0.58 ft	0.57 ft	5.22 ft	3.59 ft	2.74 ft	2.21 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	258.2 psf	257.0 psf	255.8 psf	254.7 psf	241.9 psf	241.2 psf	240.5 psf	239.8 psf	263.3 psf	261.9 psf	260.6 psf	259.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	410.0 psf	404.2 psf	398.7 psf	393.5 psf	446.6 psf	439.6 psf	433.1 psf	426.9 psf	519.4 psf	510.2 psf	501.6 psf	493.5 psf	232.9 psf	48.9 psf	43.0 psf	43.4 psf

Maximum Bearing Pressure = 519 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 32in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

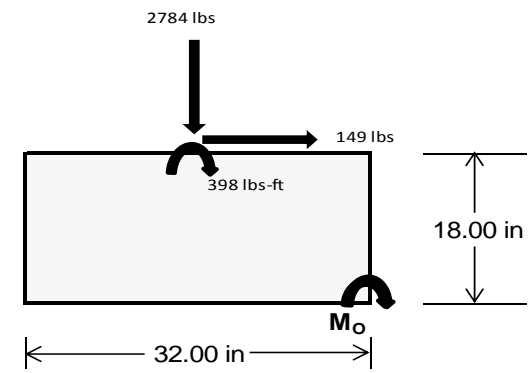
### Overturning Check

$M_o = 3090.5 \text{ ft-lbs}$   
 Resisting Force Required = 2317.88 lbs  
 S.F. = 1.67  
 Weight Required = 3863.13 lbs  
 Minimum Width = 32 in  
 Weight Provided = 6380.00 lbs

*A minimum 132in long x 32in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	32 in			32 in			32 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_Y$	247 lbs	630 lbs	218 lbs	928 lbs	2784 lbs	905 lbs	82 lbs	184 lbs	54 lbs
$F_V$	207 lbs	204 lbs	209 lbs	154 lbs	149 lbs	161 lbs	207 lbs	205 lbs	208 lbs
$P_{\text{total}}$	8146 lbs	8528 lbs	8117 lbs	8447 lbs	10303 lbs	8424 lbs	2392 lbs	2494 lbs	2363 lbs
$M$	827 lbs-ft	821 lbs-ft	832 lbs-ft	627 lbs-ft	621 lbs-ft	648 lbs-ft	824 lbs-ft	818 lbs-ft	825 lbs-ft
$e$	0.10 ft	0.10 ft	0.10 ft	0.07 ft	0.06 ft	0.08 ft	0.34 ft	0.33 ft	0.35 ft
$L/6$	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft
$f_{\text{min}}$	214.3 psf	227.8 psf	212.9 psf	239.9 psf	303.6 psf	237.5 psf	18.4 psf	22.3 psf	17.3 psf
$f_{\text{max}}$	341.1 psf	353.7 psf	340.5 psf	336.0 psf	398.9 psf	336.9 psf	144.7 psf	147.8 psf	143.9 psf



Maximum Bearing Pressure = 399 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 32in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 32in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

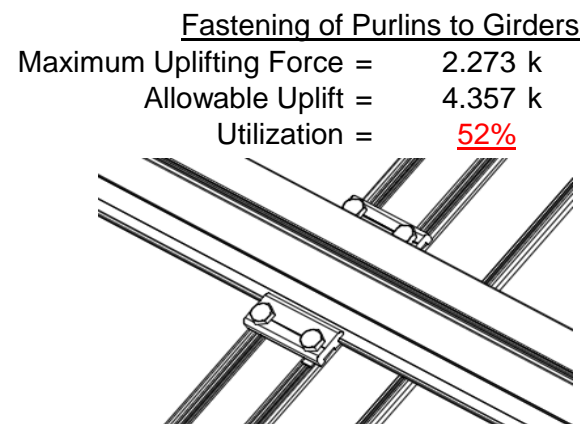
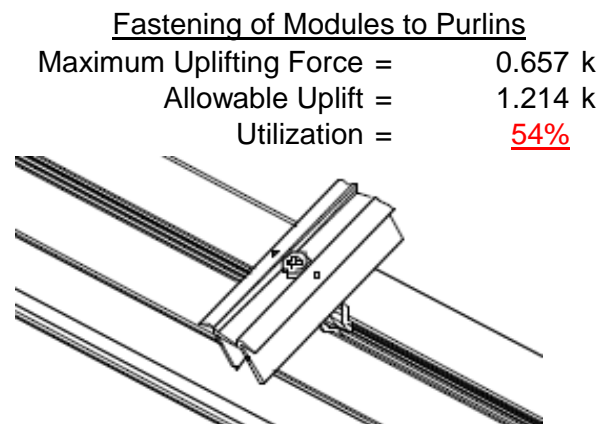
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

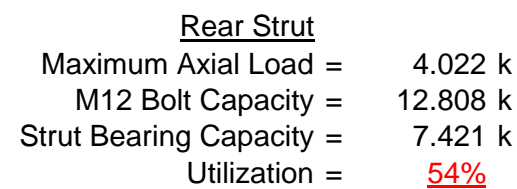
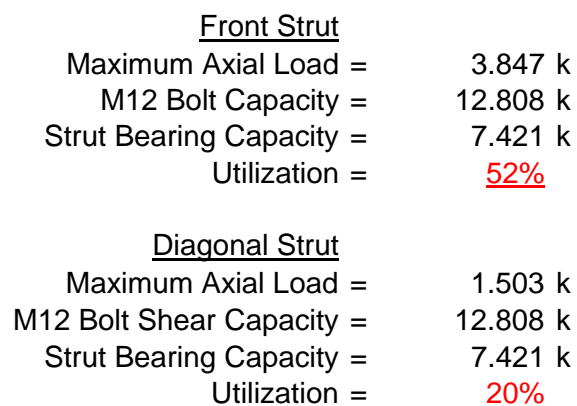
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



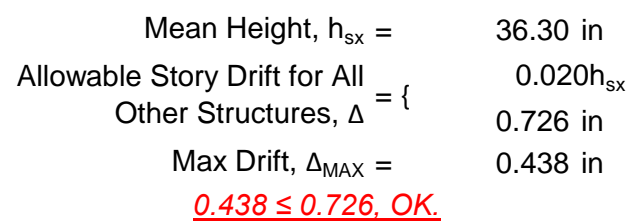
Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 117 \text{ in}$$

$$J = 0.432$$

$$323.677$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 117$$

$$J = 0.432$$

$$205.839$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.7$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-52.98	-52.98	0	0
2	M14	y	-52.98	-52.98	0	0
3	M15	y	-84.769	-84.769	0	0
4	M16	y	-84.769	-84.769	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	121.855	121.855	0	0
2	M14	y	94.305	94.305	0	0
3	M15	y	52.98	52.98	0	0
4	M16	y	52.98	52.98	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	79.576	1	909.777	1	-4.195	12	.005	14	.243	1	1.824	1
20		min	3.55	12	-1156.811	3	-143.858	1	-.015	1	.005	12	-2.258	3
21	11	max	79.576	1	749.18	1	-3.193	12	.015	1	.104	1	.926	1
22		min	3.55	12	-951.203	3	-113.129	1	0	3	.001	12	-1.116	3
23	12	max	79.576	1	588.583	1	-2.191	12	.015	1	.033	4	.201	1
24		min	3.55	12	-745.596	3	-82.401	1	0	3	-.002	3	-.197	3
25	13	max	79.576	1	427.986	1	-1.19	12	.015	1	.015	5	.499	3
26		min	3.55	12	-539.988	3	-51.672	1	0	3	-.074	1	-.35	1
27	14	max	79.576	1	267.389	1	-.188	12	.015	1	0	15	.973	3
28		min	.614	15	-334.38	3	-20.943	1	0	3	-.114	1	-.726	1
29	15	max	79.576	1	106.792	1	9.786	1	.015	1	-.004	12	1.224	3
30		min	-10.102	5	-128.772	3	-12.635	5	0	3	-.12	1	-.929	1
31	16	max	79.576	1	76.836	3	40.514	1	.015	1	-.002	12	1.252	3
32		min	-21.223	5	-53.805	1	-11.085	5	0	3	-.092	1	-.958	1
33	17	max	79.576	1	282.444	3	71.243	1	.015	1	0	3	1.057	3
34		min	-32.343	5	-214.402	1	-9.535	5	0	3	-.047	4	-.812	1
35	18	max	79.576	1	488.052	3	101.972	1	.015	1	.062	1	.64	3
36		min	-43.464	5	-374.999	1	-7.985	5	0	3	-.049	5	-.493	1
37	19	max	79.576	1	693.659	3	132.7	1	.015	1	.189	1	0	1
38		min	-54.585	5	-535.596	1	-6.436	5	0	3	-.057	5	0	3
39	M14	1	max	59.314	4	569.395	1	-4.952	12	.009	.216	1	0	1
40		min	1.518	12	-549.682	3	-136.893	1	-.012	1	.009	12	0	3
41	2	max	48.194	4	408.798	1	-3.951	12	.009	3	.099	4	.51	3
42		min	1.518	12	-391.828	3	-106.165	1	-.012	1	.005	12	-.53	1
43	3	max	37.68	1	248.201	1	-2.949	12	.009	3	.054	5	.849	3
44		min	1.518	12	-233.973	3	-75.436	1	-.012	1	-.014	1	-.886	1
45	4	max	37.68	1	87.604	1	-1.947	12	.009	3	.029	5	1.017	3
46		min	1.518	12	-76.118	3	-44.707	1	-.012	1	-.079	1	-1.068	1
47	5	max	37.68	1	81.736	3	-.942	10	.009	3	.006	5	1.014	3
48		min	1.518	12	-72.993	1	-23.952	4	-.012	1	-.111	1	-1.076	1
49	6	max	37.68	1	239.591	3	16.75	1	.009	3	-.004	12	.84	3
50		min	-3.985	5	-233.591	1	-19.085	5	-.012	1	-.109	1	-.909	1
51	7	max	37.68	1	397.445	3	47.479	1	.009	3	-.003	12	.495	3
52		min	-15.106	5	-394.188	1	-17.535	5	-.012	1	-.074	1	-.569	1
53	8	max	37.68	1	555.3	3	78.208	1	.009	3	0	10	0	15
54		min	-26.226	5	-554.785	1	-15.986	5	-.012	1	-.056	4	-.064	2
55	9	max	37.68	1	713.154	3	108.936	1	.009	3	.095	1	.633	1
56		min	-37.347	5	-715.382	1	-14.436	5	-.012	1	-.07	5	-.708	3
57	10	max	59.579	4	875.979	1	-4.063	12	.009	3	.23	1	1.495	1
58		min	1.518	12	-871.009	3	-139.665	1	-.012	1	.005	12	-1.566	3
59	11	max	48.459	4	715.382	1	-3.061	12	.012	1	.1	4	.633	1
60		min	1.518	12	-713.154	3	-108.936	1	-.009	3	.001	12	-.708	3
61	12	max	37.68	1	554.785	1	-2.059	12	.012	1	.053	5	0	15
62		min	1.518	12	-555.3	3	-78.208	1	-.009	3	-.006	1	-.064	2
63	13	max	37.68	1	394.188	1	-1.058	12	.012	1	.028	5	.495	3
64		min	1.518	12	-397.445	3	-47.479	1	-.009	3	-.074	1	-.569	1
65	14	max	37.68	1	233.591	1	-.037	3	.012	1	.005	5	.84	3
66		min	1.518	12	-239.591	3	-24.508	4	-.009	3	-.109	1	-.909	1
67	15	max	37.68	1	72.993	1	13.978	1	.012	1	-.003	12	1.014	3
68		min	-3.492	5	-81.736	3	-19.198	5	-.009	3	-.111	1	-1.076	1
69	16	max	37.68	1	76.118	3	44.707	1	.012	1	-.002	12	1.017	3
70		min	-14.613	5	-87.604	1	-17.648	5	-.009	3	-.079	1	-1.068	1
71	17	max	37.68	1	233.973	3	75.436	1	.012	1	.001	3	.849	3
72		min	-25.733	5	-248.201	1	-16.098	5	-.009	3	-.059	4	-.886	1
73	18	max	37.68	1	391.828	3	106.165	1	.012	1	.085	1	.51	3
74		min	-36.854	5	-408.798	1	-14.548	5	-.009	3	-.072	5	-.53	1
75	19	max	37.68	1	549.682	3	136.893	1	.012	1	.216	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-47.974	5	-569.395	1	-12.999	5	-.009	3	-.087	5	0	3
77	M15	1	max	76.906	5	671.185	2	-4.915	12	.013	1	.216	1	0	2
78			min	-39.48	1	-308.204	3	-136.881	1	-.008	3	.009	12	0	3
79		2	max	65.786	5	479.893	2	-3.913	12	.013	1	.135	4	.287	3
80			min	-39.48	1	-221.98	3	-106.152	1	-.008	3	.004	12	-.623	2
81		3	max	54.665	5	288.602	2	-2.912	12	.013	1	.079	5	.481	3
82			min	-39.48	1	-135.755	3	-75.423	1	-.008	3	-.014	1	-1.04	2
83		4	max	43.544	5	97.94	1	-1.91	12	.013	1	.045	5	.581	3
84			min	-39.48	1	-49.531	3	-44.695	1	-.008	3	-.079	1	-1.249	2
85		5	max	32.424	5	36.694	3	-.908	12	.013	1	.012	5	.588	3
86			min	-39.48	1	-93.982	2	-33.069	4	-.008	3	-.111	1	-1.251	2
87		6	max	21.303	5	122.918	3	16.763	1	.013	1	-.004	12	.502	3
88			min	-39.48	1	-285.273	2	-28.2	5	-.008	3	-.109	1	-1.045	2
89		7	max	10.182	5	209.143	3	47.492	1	.013	1	-.003	12	.322	3
90			min	-39.48	1	-476.565	2	-26.65	5	-.008	3	-.074	1	-.637	1
91		8	max	-.59	15	295.367	3	78.22	1	.013	1	0	10	.049	3
92			min	-39.48	1	-667.856	2	-25.1	5	-.008	3	-.08	4	-.029	1
93		9	max	-1.815	12	381.592	3	108.949	1	.013	1	.095	1	.815	2
94			min	-39.48	1	-859.148	2	-23.55	5	-.008	3	-.104	5	-.318	3
95		10	max	-1.815	12	1050.44	2	-4.1	12	.013	1	.23	1	1.849	2
96			min	-39.48	1	-467.816	3	-139.678	1	-.008	11	.005	12	-.778	3
97		11	max	7.885	5	859.148	2	-3.098	12	.008	3	.134	4	.815	2
98			min	-39.48	1	-381.592	3	-108.949	1	-.013	1	.001	12	-.318	3
99		12	max	-1.815	12	667.856	2	-2.097	12	.008	3	.077	5	.049	3
100			min	-39.48	1	-295.367	3	-78.22	1	-.013	1	-.006	1	-.029	1
101		13	max	-1.815	12	476.565	2	-1.095	12	.008	3	.042	5	.322	3
102			min	-39.48	1	-209.143	3	-47.492	1	-.013	1	-.074	1	-.637	1
103		14	max	-1.815	12	285.273	2	-.093	12	.008	3	.009	5	.502	3
104			min	-39.48	1	-122.918	3	-33.638	4	-.013	1	-.109	1	-1.045	2
105		15	max	-1.815	12	93.982	2	13.966	1	.008	3	-.003	12	.588	3
106			min	-45.038	4	-36.694	3	-28.313	5	-.013	1	-.111	1	-1.251	2
107		16	max	-1.815	12	49.531	3	44.695	1	.008	3	-.002	12	.581	3
108			min	-56.159	4	-97.94	1	-26.763	5	-.013	1	-.079	1	-1.249	2
109		17	max	-1.815	12	135.755	3	75.423	1	.008	3	.001	3	.481	3
110			min	-67.279	4	-288.602	2	-25.214	5	-.013	1	-.085	4	-1.04	2
111		18	max	-1.815	12	221.98	3	106.152	1	.008	3	.084	1	.287	3
112			min	-78.4	4	-479.893	2	-23.664	5	-.013	1	-.107	5	-.623	2
113		19	max	-1.815	12	308.204	3	136.881	1	.008	3	.216	1	0	2
114			min	-89.521	4	-671.185	2	-22.114	5	-.013	1	-.132	5	0	5
115	M16	1	max	76.671	5	640.568	2	-4.695	12	.013	1	.19	1	0	2
116			min	-84.046	1	-286.275	3	-132.894	1	-.011	3	.008	12	0	3
117		2	max	65.55	5	449.277	2	-3.693	12	.013	1	.099	4	.263	3
118			min	-84.046	1	-200.051	3	-102.166	1	-.011	3	.003	12	-.59	2
119		3	max	54.429	5	257.985	2	-2.691	12	.013	1	.058	5	.433	3
120			min	-84.046	1	-113.826	3	-71.437	1	-.011	3	-.031	1	-.973	2
121		4	max	43.309	5	66.694	2	-1.69	12	.013	1	.033	5	.51	3
122			min	-84.046	1	-27.602	3	-40.708	1	-.011	3	-.092	1	-1.149	2
123		5	max	32.188	5	58.623	3	-.652	10	.013	1	.009	5	.493	3
124			min	-84.046	1	-124.598	2	-23.56	4	-.011	3	-.119	1	-1.118	2
125		6	max	21.067	5	144.847	3	20.749	1	.013	1	-.004	12	.383	3
126			min	-84.046	1	-315.89	2	-19.63	5	-.011	3	-.114	1	-.879	2
127		7	max	9.947	5	231.072	3	51.478	1	.013	1	-.003	12	.179	3
128			min	-84.046	1	-507.181	2	-18.081	5	-.011	3	-.074	1	-.433	2
129		8	max	-.731	15	317.296	3	82.207	1	.013	1	0	10	.223	1
130			min	-84.046	1	-698.473	2	-16.531	5	-.011	3	-.054	4	-.118	3
131		9	max	-3.519	12	403.521	3	112.935	1	.013	1	.104	1	1.08	2
132			min	-84.046	1	-889.765	2	-14.981	5	-.011	3	-.07	5	-.508	3





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-3.519	12	1081.056	2	-4.321	12	.013	1	.243	1	2.147	2
134			min	-84.046	1	-489.746	3	-143.664	1	-.011	3	.006	12	-.992	3
135		11	max	3.825	5	889.765	2	-3.319	12	.011	3	.104	1	1.08	2
136			min	-84.046	1	-403.521	3	-112.935	1	-.013	1	.002	12	-.508	3
137		12	max	-3.519	12	698.473	2	-2.317	12	.011	3	.053	4	.223	1
138			min	-84.046	1	-317.296	3	-82.207	1	-.013	1	-.002	1	-.118	3
139		13	max	-3.519	12	507.181	2	-1.315	12	.011	3	.026	5	.179	3
140			min	-84.046	1	-231.072	3	-51.478	1	-.013	1	-.074	1	-.433	2
141		14	max	-3.519	12	315.89	2	-.314	12	.011	3	.002	5	.383	3
142			min	-84.046	1	-144.847	3	-26.15	4	-.013	1	-.114	1	-.879	2
143		15	max	-3.519	12	124.598	2	9.98	1	.011	3	-.004	12	.493	3
144			min	-84.046	1	-58.623	3	-20.156	5	-.013	1	-.119	1	-1.118	2
145		16	max	-3.519	12	27.602	3	40.708	1	.011	3	-.002	12	.51	3
146			min	-84.046	1	-66.694	2	-18.606	5	-.013	1	-.092	1	-1.149	2
147		17	max	-3.519	12	113.826	3	71.437	1	.011	3	0	3	.433	3
148			min	-84.046	1	-257.985	2	-17.056	5	-.013	1	-.068	4	-.973	2
149		18	max	-3.519	12	200.051	3	102.166	1	.011	3	.063	1	.263	3
150			min	-91.697	4	-449.277	2	-15.507	5	-.013	1	-.079	5	-.59	2
151		19	max	-3.519	12	286.275	3	132.894	1	.011	3	.19	1	0	2
152			min	-102.817	4	-640.568	2	-13.957	5	-.013	1	-.095	5	0	5
153	M2	1	max	1170.124	1	2.333	4	1.133	1	0	3	0	3	0	1
154			min	-1230.683	3	.572	15	-75.902	4	0	4	0	1	0	1
155		2	max	1170.452	1	2.318	4	1.133	1	0	3	0	1	0	15
156			min	-1230.436	3	.569	15	-76.186	4	0	4	-.017	4	0	4
157		3	max	1170.78	1	2.303	4	1.133	1	0	3	0	1	0	15
158			min	-1230.19	3	.565	15	-76.471	4	0	4	-.034	4	-.001	4
159		4	max	1171.109	1	2.288	4	1.133	1	0	3	0	1	0	15
160			min	-1229.944	3	.561	15	-76.756	4	0	4	-.051	4	-.002	4
161		5	max	1171.437	1	2.272	4	1.133	1	0	3	0	1	0	15
162			min	-1229.697	3	.558	15	-77.041	4	0	4	-.068	4	-.002	4
163		6	max	1171.766	1	2.257	4	1.133	1	0	3	.001	1	0	15
164			min	-1229.451	3	.554	15	-77.326	4	0	4	-.085	4	-.003	4
165		7	max	1172.094	1	2.242	4	1.133	1	0	3	.001	1	0	15
166			min	-1229.205	3	.551	15	-77.611	4	0	4	-.102	4	-.003	4
167		8	max	1172.423	1	2.227	4	1.133	1	0	3	.002	1	0	15
168			min	-1228.958	3	.547	15	-77.895	4	0	4	-.119	4	-.004	4
169		9	max	1172.751	1	2.211	4	1.133	1	0	3	.002	1	0	15
170			min	-1228.712	3	.544	15	-78.18	4	0	4	-.137	4	-.004	4
171		10	max	1173.079	1	2.196	4	1.133	1	0	3	.002	1	-.001	15
172			min	-1228.466	3	.54	15	-78.465	4	0	4	-.154	4	-.005	4
173		11	max	1173.408	1	2.181	4	1.133	1	0	3	.002	1	-.001	15
174			min	-1228.219	3	.536	15	-78.75	4	0	4	-.171	4	-.005	4
175		12	max	1173.736	1	2.166	4	1.133	1	0	3	.003	1	-.001	15
176			min	-1227.973	3	.533	15	-79.035	4	0	4	-.189	4	-.005	4
177		13	max	1174.065	1	2.15	4	1.133	1	0	3	.003	1	-.001	15
178			min	-1227.727	3	.529	15	-79.319	4	0	4	-.206	4	-.006	4
179		14	max	1174.393	1	2.135	4	1.133	1	0	3	.003	1	-.002	15
180			min	-1227.48	3	.526	15	-79.604	4	0	4	-.224	4	-.006	4
181		15	max	1174.722	1	2.12	4	1.133	1	0	3	.003	1	-.002	15
182			min	-1227.234	3	.522	15	-79.889	4	0	4	-.242	4	-.007	4
183		16	max	1175.05	1	2.105	4	1.133	1	0	3	.004	1	-.002	15
184			min	-1226.988	3	.518	15	-80.174	4	0	4	-.259	4	-.007	4
185		17	max	1175.379	1	2.089	4	1.133	1	0	3	.004	1	-.002	15
186			min	-1226.741	3	.515	15	-80.459	4	0	4	-.277	4	-.008	4
187		18	max	1175.707	1	2.074	4	1.133	1	0	3	.004	1	-.002	15
188			min	-1226.495	3	.511	15	-80.744	4	0	4	-.295	4	-.008	4
189		19	max	1176.035	1	2.059	4	1.133	1	0	3	.005	1	-.002	15



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1226.249	3	.508	15	-81.028	4	0	4	-.313	4	-.009	4
191	M3	1	max	358.935	2	8.106	4	.015	1	0	3	0	.009	4
192		min	-476.885	3	1.918	15	-1.186	5	0	4	-.012	4	.002	15
193		2	max	358.764	2	7.333	4	.015	1	0	3	0	.006	4
194		min	-477.013	3	1.736	15	-.644	5	0	4	-.012	4	.001	12
195		3	max	358.594	2	6.561	4	.015	14	0	3	0	.003	2
196		min	-477.14	3	1.555	15	-.101	5	0	4	-.012	4	0	3
197		4	max	358.424	2	5.788	4	.494	4	0	3	0	0	2
198		min	-477.268	3	1.373	15	0	12	0	4	-.012	4	-.001	3
199		5	max	358.253	2	5.016	4	1.036	4	0	3	0	0	15
200		min	-477.396	3	1.192	15	0	12	0	4	-.012	4	-.003	3
201		6	max	358.083	2	4.244	4	1.578	4	0	3	0	0	15
202		min	-477.524	3	1.01	15	0	12	0	4	-.011	4	-.004	6
203		7	max	357.913	2	3.471	4	2.12	4	0	3	0	1	15
204		min	-477.651	3	.828	15	0	12	0	4	-.011	4	-.006	6
205		8	max	357.742	2	2.699	4	2.662	4	0	3	0	1	15
206		min	-477.779	3	.647	15	0	12	0	4	-.01	4	-.007	6
207		9	max	357.572	2	1.926	4	3.204	4	0	3	0	1	15
208		min	-477.907	3	.465	15	0	12	0	4	-.008	5	-.008	6
209		10	max	357.402	2	1.154	4	3.746	4	0	3	0	1	15
210		min	-478.035	3	.284	15	0	12	0	4	-.007	5	-.009	6
211		11	max	357.231	2	.429	2	4.288	4	0	3	0	1	15
212		min	-478.162	3	-.004	3	0	12	0	4	-.005	5	-.009	6
213		12	max	357.061	2	-.079	15	4.831	4	0	3	0	1	15
214		min	-478.29	3	-.455	3	0	12	0	4	-.003	5	-.009	6
215		13	max	356.891	2	-.261	15	5.373	4	0	3	0	1	15
216		min	-478.418	3	-1.165	6	0	12	0	4	-.001	5	-.009	6
217		14	max	356.72	2	-.443	15	5.915	4	0	3	.001	4	15
218		min	-478.546	3	-1.937	6	0	12	0	4	0	12	-.008	6
219		15	max	356.55	2	-.624	15	6.457	4	0	3	.004	4	15
220		min	-478.674	3	-2.709	6	0	12	0	4	0	12	-.007	6
221		16	max	356.38	2	-.806	15	6.999	4	0	3	.007	4	15
222		min	-478.801	3	-3.482	6	0	12	0	4	0	12	-.006	6
223		17	max	356.209	2	-.987	15	7.541	4	0	3	.01	4	15
224		min	-478.929	3	-4.254	6	0	12	0	4	0	12	-.004	6
225		18	max	356.039	2	-1.169	15	8.083	4	0	3	.013	4	15
226		min	-479.057	3	-5.027	6	0	12	0	4	0	12	-.002	6
227		19	max	355.869	2	-1.35	15	8.625	4	0	3	.017	4	1
228		min	-479.185	3	-5.799	6	0	12	0	4	0	12	0	1
229	M4	1	max	1299.985	1	0	1	-.314	12	0	1	.009	4	1
230		min	-384.414	3	0	1	-230.567	4	0	1	0	10	0	1
231		2	max	1300.155	1	0	1	-.314	12	0	1	0	12	1
232		min	-384.286	3	0	1	-230.714	4	0	1	-.018	4	0	1
233		3	max	1300.326	1	0	1	-.314	12	0	1	0	12	1
234		min	-384.158	3	0	1	-230.862	4	0	1	-.044	4	0	1
235		4	max	1300.496	1	0	1	-.314	12	0	1	0	12	1
236		min	-384.03	3	0	1	-231.01	4	0	1	-.071	4	0	1
237		5	max	1300.666	1	0	1	-.314	12	0	1	0	12	1
238		min	-383.903	3	0	1	-231.157	4	0	1	-.097	4	0	1
239		6	max	1300.837	1	0	1	-.314	12	0	1	0	12	1
240		min	-383.775	3	0	1	-231.305	4	0	1	-.124	4	0	1
241		7	max	1301.007	1	0	1	-.314	12	0	1	0	12	1
242		min	-383.647	3	0	1	-231.453	4	0	1	-.15	4	0	1
243		8	max	1301.177	1	0	1	-.314	12	0	1	0	12	1
244		min	-383.519	3	0	1	-231.6	4	0	1	-.177	4	0	1
245		9	max	1301.348	1	0	1	-.314	12	0	1	0	12	1
246		min	-383.392	3	0	1	-231.748	4	0	1	-.204	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1301.518	1	0	1	-.314	12	0	1	0	12	0	1
248		min	-383.264	3	0	1	-231.896	4	0	1	-.23	4	0	1
249	11	max	1301.688	1	0	1	-.314	12	0	1	0	12	0	1
250		min	-383.136	3	0	1	-232.043	4	0	1	-.257	4	0	1
251	12	max	1301.859	1	0	1	-.314	12	0	1	0	12	0	1
252		min	-383.008	3	0	1	-232.191	4	0	1	-.283	4	0	1
253	13	max	1302.029	1	0	1	-.314	12	0	1	0	12	0	1
254		min	-382.88	3	0	1	-232.338	4	0	1	-.31	4	0	1
255	14	max	1302.199	1	0	1	-.314	12	0	1	0	12	0	1
256		min	-382.753	3	0	1	-232.486	4	0	1	-.337	4	0	1
257	15	max	1302.37	1	0	1	-.314	12	0	1	0	12	0	1
258		min	-382.625	3	0	1	-232.634	4	0	1	-.363	4	0	1
259	16	max	1302.54	1	0	1	-.314	12	0	1	0	12	0	1
260		min	-382.497	3	0	1	-232.781	4	0	1	-.39	4	0	1
261	17	max	1302.71	1	0	1	-.314	12	0	1	0	12	0	1
262		min	-382.369	3	0	1	-232.929	4	0	1	-.417	4	0	1
263	18	max	1302.881	1	0	1	-.314	12	0	1	0	12	0	1
264		min	-382.242	3	0	1	-233.077	4	0	1	-.444	4	0	1
265	19	max	1303.051	1	0	1	-.314	12	0	1	0	12	0	1
266		min	-382.114	3	0	1	-233.224	4	0	1	-.47	4	0	1
267	M6	1	max	3759.883	1	2.848	2	0	1	0	0	4	0	1
268		min	-4022.444	3	.047	3	-76.571	4	0	4	0	1	0	1
269	2	max	3760.212	1	2.837	2	0	1	0	1	0	1	0	3
270		min	-4022.198	3	.038	3	-76.856	4	0	4	-.017	4	0	2
271	3	max	3760.54	1	2.825	2	0	1	0	1	0	1	0	3
272		min	-4021.951	3	.029	3	-77.141	4	0	4	-.034	4	-.001	2
273	4	max	3760.869	1	2.813	2	0	1	0	1	0	1	0	3
274		min	-4021.705	3	.02	3	-77.426	4	0	4	-.051	4	-.002	2
275	5	max	3761.197	1	2.801	2	0	1	0	1	0	1	0	3
276		min	-4021.459	3	.011	3	-77.71	4	0	4	-.068	4	-.003	2
277	6	max	3761.525	1	2.789	2	0	1	0	1	0	1	0	3
278		min	-4021.212	3	.002	3	-77.995	4	0	4	-.086	4	-.003	2
279	7	max	3761.854	1	2.777	2	0	1	0	1	0	1	0	3
280		min	-4020.966	3	-.007	3	-78.28	4	0	4	-.103	4	-.004	2
281	8	max	3762.182	1	2.765	2	0	1	0	1	0	1	0	3
282		min	-4020.72	3	-.016	3	-78.565	4	0	4	-.12	4	-.004	2
283	9	max	3762.511	1	2.753	2	0	1	0	1	0	1	0	3
284		min	-4020.473	3	-.025	3	-78.85	4	0	4	-.138	4	-.005	2
285	10	max	3762.839	1	2.741	2	0	1	0	1	0	1	0	3
286		min	-4020.227	3	-.034	3	-79.135	4	0	4	-.155	4	-.006	2
287	11	max	3763.168	1	2.73	2	0	1	0	1	0	1	0	3
288		min	-4019.981	3	-.042	3	-79.419	4	0	4	-.173	4	-.006	2
289	12	max	3763.496	1	2.718	2	0	1	0	1	0	1	0	3
290		min	-4019.734	3	-.051	3	-79.704	4	0	4	-.19	4	-.007	2
291	13	max	3763.824	1	2.706	2	0	1	0	1	0	1	0	3
292		min	-4019.488	3	-.06	3	-79.989	4	0	4	-.208	4	-.007	2
293	14	max	3764.153	1	2.694	2	0	1	0	1	0	1	0	3
294		min	-4019.242	3	-.069	3	-80.274	4	0	4	-.226	4	-.008	2
295	15	max	3764.481	1	2.682	2	0	1	0	1	0	1	0	3
296		min	-4018.995	3	-.078	3	-80.559	4	0	4	-.244	4	-.009	2
297	16	max	3764.81	1	2.67	2	0	1	0	1	0	1	0	3
298		min	-4018.749	3	-.087	3	-80.844	4	0	4	-.262	4	-.009	2
299	17	max	3765.138	1	2.658	2	0	1	0	1	0	1	0	3
300		min	-4018.503	3	-.096	3	-81.128	4	0	4	-.279	4	-.01	2
301	18	max	3765.467	1	2.646	2	0	1	0	1	0	1	0	3
302		min	-4018.256	3	-.105	3	-81.413	4	0	4	-.297	4	-.01	2
303	19	max	3765.795	1	2.634	2	0	1	0	1	0	1	0	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-4018.01	3	-1.114	3	-81.698	4	0	4	-.316	4	-.011	2
305	M7	1	max	1395.412	2	8.117	6	0	1	0	0	1	.011	2
306		min	-1501.095	3	1.905	15	-1.256	5	0	4	-.012	4	0	3
307		2	max	1395.242	2	7.344	6	0	1	0	0	1	.008	2
308		min	-1501.223	3	1.723	15	-.714	5	0	4	-.012	4	-.002	3
309		3	max	1395.071	2	6.572	6	0	1	0	0	1	.006	2
310		min	-1501.351	3	1.541	15	-.172	5	0	4	-.012	4	-.003	3
311		4	max	1394.901	2	5.799	6	.415	4	0	0	1	.003	2
312		min	-1501.479	3	1.36	15	0	1	0	4	-.012	4	-.004	3
313		5	max	1394.731	2	5.027	6	.957	4	0	0	1	.001	2
314		min	-1501.606	3	1.178	15	0	1	0	4	-.012	4	-.005	3
315		6	max	1394.56	2	4.255	6	1.5	4	0	0	1	0	2
316		min	-1501.734	3	.997	15	0	1	0	4	-.012	4	-.006	3
317		7	max	1394.39	2	3.482	6	2.042	4	0	0	1	-.001	15
318		min	-1501.862	3	.815	15	0	1	0	4	-.011	4	-.007	3
319		8	max	1394.22	2	2.71	6	2.584	4	0	0	1	-.002	15
320		min	-1501.99	3	.618	12	0	1	0	4	-.01	4	-.007	3
321		9	max	1394.049	2	2.042	2	3.126	4	0	0	1	-.002	15
322		min	-1502.118	3	.318	12	0	1	0	4	-.009	4	-.008	4
323		10	max	1393.879	2	1.44	2	3.668	4	0	0	1	-.002	15
324		min	-1502.245	3	-.018	3	0	1	0	4	-.007	4	-.009	4
325		11	max	1393.709	2	.838	2	4.21	4	0	0	1	-.002	15
326		min	-1502.373	3	-.469	3	0	1	0	4	-.006	4	-.009	4
327		12	max	1393.538	2	.237	2	4.752	4	0	0	1	-.002	15
328		min	-1502.501	3	-.92	3	0	1	0	4	-.004	4	-.009	4
329		13	max	1393.368	2	-.274	15	5.294	4	0	0	1	-.002	15
330		min	-1502.629	3	-1.372	3	0	1	0	4	-.002	5	-.009	4
331		14	max	1393.198	2	-.456	15	5.837	4	0	0	4	-.002	15
332		min	-1502.756	3	-1.925	4	0	1	0	4	0	1	-.008	4
333		15	max	1393.027	2	-.637	15	6.379	4	0	.003	4	-.002	15
334		min	-1502.884	3	-2.697	4	0	1	0	4	0	1	-.007	4
335		16	max	1392.857	2	-.819	15	6.921	4	0	.006	4	-.001	15
336		min	-1503.012	3	-3.47	4	0	1	0	4	0	1	-.006	4
337		17	max	1392.687	2	-1.001	15	7.463	4	0	.009	4	0	15
338		min	-1503.14	3	-4.242	4	0	1	0	4	0	1	-.004	4
339		18	max	1392.516	2	-1.182	15	8.005	4	0	.012	4	0	15
340		min	-1503.267	3	-5.014	4	0	1	0	4	0	1	-.002	4
341		19	max	1392.346	2	-1.364	15	8.547	4	0	.016	4	0	1
342		min	-1503.395	3	-5.787	4	0	1	0	4	0	1	0	1
343	M8	1	max	3843.851	1	0	1	0	1	0	.008	4	0	1
344		min	-1302.047	3	0	1	-225.488	4	0	1	0	1	0	1
345		2	max	3844.021	1	0	1	0	1	0	0	1	0	1
346		min	-1301.919	3	0	1	-225.636	4	0	1	-.017	4	0	1
347		3	max	3844.192	1	0	1	0	1	0	0	1	0	1
348		min	-1301.791	3	0	1	-225.783	4	0	1	-.043	4	0	1
349		4	max	3844.362	1	0	1	0	1	0	0	1	0	1
350		min	-1301.663	3	0	1	-225.931	4	0	1	-.069	4	0	1
351		5	max	3844.532	1	0	1	0	1	0	0	1	0	1
352		min	-1301.536	3	0	1	-226.078	4	0	1	-.095	4	0	1
353		6	max	3844.703	1	0	1	0	1	0	0	1	0	1
354		min	-1301.408	3	0	1	-226.226	4	0	1	-.121	4	0	1
355		7	max	3844.873	1	0	1	0	1	0	0	1	0	1
356		min	-1301.28	3	0	1	-226.374	4	0	1	-.147	4	0	1
357		8	max	3845.043	1	0	1	0	1	0	0	1	0	1
358		min	-1301.152	3	0	1	-226.521	4	0	1	-.173	4	0	1
359		9	max	3845.214	1	0	1	0	1	0	0	1	0	1
360		min	-1301.025	3	0	1	-226.669	4	0	1	-.199	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3845.384	1	0	1	0	1	0	1	0	1	0	1
362			min	-1300.897	3	0	1	-226.817	4	0	1	-.225	4	0	1
363		11	max	3845.554	1	0	1	0	1	0	1	0	1	0	1
364			min	-1300.769	3	0	1	-226.964	4	0	1	-.251	4	0	1
365		12	max	3845.725	1	0	1	0	1	0	1	0	1	0	1
366			min	-1300.641	3	0	1	-227.112	4	0	1	-.277	4	0	1
367		13	max	3845.895	1	0	1	0	1	0	1	0	1	0	1
368			min	-1300.514	3	0	1	-227.26	4	0	1	-.303	4	0	1
369		14	max	3846.065	1	0	1	0	1	0	1	0	1	0	1
370			min	-1300.386	3	0	1	-227.407	4	0	1	-.33	4	0	1
371		15	max	3846.236	1	0	1	0	1	0	1	0	1	0	1
372			min	-1300.258	3	0	1	-227.555	4	0	1	-.356	4	0	1
373		16	max	3846.406	1	0	1	0	1	0	1	0	1	0	1
374			min	-1300.13	3	0	1	-227.702	4	0	1	-.382	4	0	1
375		17	max	3846.576	1	0	1	0	1	0	1	0	1	0	1
376			min	-1300.003	3	0	1	-227.85	4	0	1	-.408	4	0	1
377		18	max	3846.747	1	0	1	0	1	0	1	0	1	0	1
378			min	-1299.875	3	0	1	-227.998	4	0	1	-.434	4	0	1
379		19	max	3846.917	1	0	1	0	1	0	1	0	1	0	1
380			min	-1299.747	3	0	1	-228.145	4	0	1	-.46	4	0	1
381	M10	1	max	1170.124	1	2.229	6	-.046	12	0	1	0	1	0	1
382			min	-1230.683	3	.502	15	-76.471	4	0	5	0	3	0	1
383		2	max	1170.452	1	2.214	6	-.046	12	0	1	0	10	0	15
384			min	-1230.436	3	.499	15	-76.756	4	0	5	-.017	4	0	6
385		3	max	1170.78	1	2.199	6	-.046	12	0	1	0	12	0	15
386			min	-1230.19	3	.495	15	-77.041	4	0	5	-.034	4	0	6
387		4	max	1171.109	1	2.184	6	-.046	12	0	1	0	12	0	15
388			min	-1229.944	3	.492	15	-77.326	4	0	5	-.051	4	-.001	6
389		5	max	1171.437	1	2.168	6	-.046	12	0	1	0	12	0	15
390			min	-1229.697	3	.488	15	-77.61	4	0	5	-.068	4	-.002	6
391		6	max	1171.766	1	2.153	6	-.046	12	0	1	0	12	0	15
392			min	-1229.451	3	.484	15	-77.895	4	0	5	-.085	4	-.002	6
393		7	max	1172.094	1	2.138	6	-.046	12	0	1	0	12	0	15
394			min	-1229.205	3	.481	15	-78.18	4	0	5	-.103	4	-.003	6
395		8	max	1172.423	1	2.123	6	-.046	12	0	1	0	12	0	15
396			min	-1228.958	3	.477	15	-78.465	4	0	5	-.12	4	-.003	6
397		9	max	1172.751	1	2.107	6	-.046	12	0	1	0	12	0	15
398			min	-1228.712	3	.474	15	-78.75	4	0	5	-.138	4	-.004	6
399		10	max	1173.079	1	2.092	6	-.046	12	0	1	0	12	0	15
400			min	-1228.466	3	.47	15	-79.035	4	0	5	-.155	4	-.004	6
401		11	max	1173.408	1	2.077	6	-.046	12	0	1	0	12	-.001	15
402			min	-1228.219	3	.466	15	-79.319	4	0	5	-.173	4	-.005	6
403		12	max	1173.736	1	2.062	6	-.046	12	0	1	0	12	-.001	15
404			min	-1227.973	3	.463	15	-79.604	4	0	5	-.19	4	-.005	6
405		13	max	1174.065	1	2.046	6	-.046	12	0	1	0	12	-.001	15
406			min	-1227.727	3	.459	15	-79.889	4	0	5	-.208	4	-.006	6
407		14	max	1174.393	1	2.031	6	-.046	12	0	1	0	12	-.001	15
408			min	-1227.48	3	.456	15	-80.174	4	0	5	-.226	4	-.006	6
409		15	max	1174.722	1	2.016	6	-.046	12	0	1	0	12	-.001	15
410			min	-1227.234	3	.452	15	-80.459	4	0	5	-.243	4	-.007	6
411		16	max	1175.05	1	2	6	-.046	12	0	1	0	12	-.002	15
412			min	-1226.988	3	.448	15	-80.744	4	0	5	-.261	4	-.007	6
413		17	max	1175.379	1	1.985	6	-.046	12	0	1	0	12	-.002	15
414			min	-1226.741	3	.445	15	-81.028	4	0	5	-.279	4	-.007	6
415		18	max	1175.707	1	1.97	6	-.046	12	0	1	0	12	-.002	15
416			min	-1226.495	3	.441	15	-81.313	4	0	5	-.297	4	-.008	6
417		19	max	1176.035	1	1.955	6	-.046	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1226.249	3	.438	15	-81.598	4	0	5	-.315	4	-.008	6
419	M11	1	max	358.935	2	8.051	6	0	12	0	1	0	12	.008	6
420			min	-476.885	3	1.881	15	-1.187	5	0	4	-.012	4	.002	15
421		2	max	358.764	2	7.279	6	0	12	0	1	0	12	.005	2
422			min	-477.013	3	1.699	15	-.645	5	0	4	-.012	4	.001	15
423		3	max	358.594	2	6.506	6	0	12	0	1	0	12	.003	2
424			min	-477.14	3	1.518	15	-.103	5	0	4	-.012	4	0	3
425		4	max	358.424	2	5.734	6	.486	4	0	1	0	12	0	2
426			min	-477.268	3	1.336	15	-.015	1	0	4	-.012	4	-.001	3
427		5	max	358.253	2	4.961	6	1.029	4	0	1	0	12	0	15
428			min	-477.396	3	1.155	15	-.015	1	0	4	-.012	4	-.003	3
429		6	max	358.083	2	4.189	6	1.571	4	0	1	0	12	-.001	15
430			min	-477.524	3	.973	15	-.015	1	0	4	-.011	4	-.005	4
431		7	max	357.913	2	3.416	6	2.113	4	0	1	0	12	-.002	15
432			min	-477.651	3	.792	15	-.015	1	0	4	-.011	4	-.006	4
433		8	max	357.742	2	2.644	6	2.655	4	0	1	0	12	-.002	15
434			min	-477.779	3	.61	15	-.015	1	0	4	-.01	4	-.007	4
435		9	max	357.572	2	1.872	6	3.197	4	0	1	0	12	-.002	15
436			min	-477.907	3	.429	15	-.015	1	0	4	-.008	4	-.008	4
437		10	max	357.402	2	1.099	6	3.739	4	0	1	0	12	-.002	15
438			min	-478.035	3	.247	15	-.015	1	0	4	-.007	4	-.009	4
439		11	max	357.231	2	.429	2	4.281	4	0	1	0	12	-.002	15
440			min	-478.162	3	-.004	3	-.015	1	0	4	-.005	4	-.009	4
441		12	max	357.061	2	-.116	15	4.823	4	0	1	0	12	-.002	15
442			min	-478.29	3	-.455	3	-.015	1	0	4	-.003	4	-.009	4
443		13	max	356.891	2	-.298	15	5.366	4	0	1	0	12	-.002	15
444			min	-478.418	3	-1.219	4	-.015	1	0	4	-.001	4	-.009	4
445		14	max	356.72	2	-.479	15	5.908	4	0	1	.001	4	-.002	15
446			min	-478.546	3	-1.992	4	-.015	1	0	4	0	1	-.008	4
447		15	max	356.55	2	-.661	15	6.45	4	0	1	.004	4	-.002	15
448			min	-478.674	3	-2.764	4	-.015	1	0	4	0	1	-.007	4
449		16	max	356.38	2	-.842	15	6.992	4	0	1	.007	4	-.001	15
450			min	-478.801	3	-3.536	4	-.015	1	0	4	0	1	-.006	4
451		17	max	356.209	2	-1.024	15	7.534	4	0	1	.01	4	-.001	15
452			min	-478.929	3	-4.309	4	-.015	1	0	4	0	1	-.004	4
453		18	max	356.039	2	-1.206	15	8.076	4	0	1	.013	4	0	15
454			min	-479.057	3	-5.081	4	-.015	1	0	4	0	1	-.002	4
455		19	max	355.869	2	-1.387	15	8.618	4	0	1	.017	4	0	1
456			min	-479.185	3	-5.854	4	-.015	1	0	4	0	1	0	1
457	M12	1	max	1299.985	1	0	1	7.579	1	0	1	.009	4	0	1
458			min	-384.414	3	0	1	-226.745	4	0	1	0	1	0	1
459		2	max	1300.155	1	0	1	7.579	1	0	1	0	1	0	1
460			min	-384.286	3	0	1	-226.893	4	0	1	-.017	4	0	1
461		3	max	1300.326	1	0	1	7.579	1	0	1	.002	1	0	1
462			min	-384.158	3	0	1	-227.041	4	0	1	-.043	4	0	1
463		4	max	1300.496	1	0	1	7.579	1	0	1	.002	1	0	1
464			min	-384.03	3	0	1	-227.188	4	0	1	-.069	4	0	1
465		5	max	1300.666	1	0	1	7.579	1	0	1	.003	1	0	1
466			min	-383.903	3	0	1	-227.336	4	0	1	-.096	4	0	1
467		6	max	1300.837	1	0	1	7.579	1	0	1	.004	1	0	1
468			min	-383.775	3	0	1	-227.483	4	0	1	-.122	4	0	1
469		7	max	1301.007	1	0	1	7.579	1	0	1	.005	1	0	1
470			min	-383.647	3	0	1	-227.631	4	0	1	-.148	4	0	1
471		8	max	1301.177	1	0	1	7.579	1	0	1	.006	1	0	1
472			min	-383.519	3	0	1	-227.779	4	0	1	-.174	4	0	1
473		9	max	1301.348	1	0	1	7.579	1	0	1	.007	1	0	1
474			min	-383.392	3	0	1	-227.926	4	0	1	-.2	4	0	1



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Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1301.518	1	0	1	7.579	1	0	1	.008	1	0	1
476			min	-383.264	3	0	1	-228.074	4	0	1	-.226	4	0	1
477		11	max	1301.688	1	0	1	7.579	1	0	1	.009	1	0	1
478			min	-383.136	3	0	1	-228.222	4	0	1	-.252	4	0	1
479		12	max	1301.859	1	0	1	7.579	1	0	1	.009	1	0	1
480			min	-383.008	3	0	1	-228.369	4	0	1	-.279	4	0	1
481		13	max	1302.029	1	0	1	7.579	1	0	1	.01	1	0	1
482			min	-382.88	3	0	1	-228.517	4	0	1	-.305	4	0	1
483		14	max	1302.199	1	0	1	7.579	1	0	1	.011	1	0	1
484			min	-382.753	3	0	1	-228.664	4	0	1	-.331	4	0	1
485		15	max	1302.37	1	0	1	7.579	1	0	1	.012	1	0	1
486			min	-382.625	3	0	1	-228.812	4	0	1	-.357	4	0	1
487		16	max	1302.54	1	0	1	7.579	1	0	1	.013	1	0	1
488			min	-382.497	3	0	1	-228.96	4	0	1	-.384	4	0	1
489		17	max	1302.71	1	0	1	7.579	1	0	1	.014	1	0	1
490			min	-382.369	3	0	1	-229.107	4	0	1	-.41	4	0	1
491		18	max	1302.881	1	0	1	7.579	1	0	1	.015	1	0	1
492			min	-382.242	3	0	1	-229.255	4	0	1	-.436	4	0	1
493		19	max	1303.051	1	0	1	7.579	1	0	1	.016	1	0	1
494			min	-382.114	3	0	1	-229.403	4	0	1	-.463	4	0	1
495	M1	1	max	132.703	1	693.641	3	54.574	5	0	1	.189	1	0	3
496			min	-6.435	5	-534.357	1	-79.503	1	0	3	-.057	5	-.015	1
497		2	max	133.074	1	692.603	3	55.815	5	0	1	.147	1	.268	1
498			min	-6.262	5	-535.741	1	-79.503	1	0	3	-.028	5	-.365	3
499		3	max	283.888	3	601.016	1	-3.5	12	0	3	.105	1	.537	1
500			min	-174.97	2	-509.501	3	-78.569	1	0	1	0	15	-.716	3
501		4	max	284.166	3	599.632	1	-3.5	12	0	3	.064	1	.22	1
502			min	-174.599	2	-510.539	3	-78.569	1	0	1	-.008	5	-.447	3
503		5	max	284.444	3	598.249	1	-3.5	12	0	3	.022	1	-.004	15
504			min	-174.229	2	-511.576	3	-78.569	1	0	1	-.016	5	-.177	3
505		6	max	284.722	3	596.865	1	-3.5	12	0	3	0	12	.093	3
506			min	-173.858	2	-512.614	3	-78.569	1	0	1	-.027	4	-.411	1
507		7	max	285.001	3	595.481	1	-3.5	12	0	3	-.003	12	.364	3
508			min	-173.487	2	-513.652	3	-78.569	1	0	1	-.061	1	-.726	1
509		8	max	285.279	3	594.098	1	-3.5	12	0	3	-.005	12	.635	3
510			min	-173.116	2	-514.689	3	-78.569	1	0	1	-.102	1	-1.04	1
511		9	max	293.032	3	46.754	2	38.079	5	0	9	.061	1	.741	3
512			min	-120.236	2	.417	15	-116.272	1	0	3	-.113	5	-1.185	1
513		10	max	293.31	3	45.371	2	39.32	5	0	9	0	10	.723	3
514			min	-119.866	2	0	5	-116.272	1	0	3	-.094	4	-1.198	1
515		11	max	293.588	3	43.987	2	40.562	5	0	9	-.003	12	.705	3
516			min	-119.495	2	-1.736	4	-116.272	1	0	3	-.084	4	-1.21	1
517		12	max	301.283	3	342.769	3	122.77	5	0	2	.101	1	.614	3
518			min	-75.274	10	-643.523	1	-76.819	1	0	3	-.165	5	-1.069	1
519		13	max	301.561	3	341.731	3	124.011	5	0	2	.06	1	.434	3
520			min	-74.965	10	-644.907	1	-76.819	1	0	3	-.1	5	-.729	1
521		14	max	301.839	3	340.693	3	125.253	5	0	2	.02	1	.254	3
522			min	-74.656	10	-646.291	1	-76.819	1	0	3	-.034	5	-.388	1
523		15	max	302.117	3	339.656	3	126.494	5	0	2	.032	5	.074	3
524			min	-74.347	10	-647.674	1	-76.819	1	0	3	-.021	1	-.047	1
525		16	max	302.395	3	338.618	3	127.735	5	0	2	.099	5	.317	2
526			min	-74.038	10	-649.058	1	-76.819	1	0	3	-.061	1	-1.105	3
527		17	max	302.673	3	337.58	3	128.977	5	0	2	.167	5	.652	2
528			min	-73.729	10	-650.441	1	-76.819	1	0	3	-.102	1	-.283	3
529		18	max	13.784	5	642.354	2	-3.519	12	0	5	.14	5	.328	2
530			min	-133.263	1	-285.277	3	-104.097	4	0	2	-.146	1	-.14	3
531		19	max	13.957	5	640.97	2	-3.519	12	0	5	.095	5	.011	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532	M5	min	-132.892	1	-286.315	3	-102.856	4	0	2	-.19	1	-.013	1
533		max	287.71	1	2313.561	3	82.986	5	0	1	0	1	.03	1
534		min	8.39	12	-1812.38	1	0	1	0	4	-.126	4	-.002	3
535		max	288.08	1	2312.523	3	84.228	5	0	1	0	1	.986	1
536		min	8.575	12	-1813.764	1	0	1	0	4	-.082	4	-1.222	3
537		max	911.245	3	1826.305	1	13.809	4	0	4	0	1	1.9	1
538		min	-618.087	2	-1625.481	3	0	1	0	1	-.039	4	-2.395	3
539		max	911.523	3	1824.921	1	15.05	4	0	4	0	1	.937	1
540		min	-617.716	2	-1626.519	3	0	1	0	1	-.031	4	-1.537	3
541		max	911.801	3	1823.538	1	16.292	4	0	4	0	1	.016	9
542	M6	min	-617.346	2	-1627.557	3	0	1	0	1	-.023	4	-.678	3
543		max	912.079	3	1822.154	1	17.533	4	0	4	0	1	.181	3
544		min	-616.975	2	-1628.594	3	0	1	0	1	-.014	5	-.988	1
545		max	912.357	3	1820.771	1	18.775	4	0	4	0	1	1.04	3
546		min	-616.604	2	-1629.632	3	0	1	0	1	-.006	5	-1.949	1
547		max	912.635	3	1819.387	1	20.016	4	0	4	.006	4	1.901	3
548		min	-616.233	2	-1630.67	3	0	1	0	1	0	1	-2.909	1
549		max	925.007	3	156.183	2	121.753	4	0	1	0	1	2.186	3
550		min	-506.935	2	.418	15	0	1	0	1	-.149	4	-3.294	1
551		max	925.285	3	154.8	2	122.995	4	0	1	0	1	2.12	3
552	M7	min	-506.564	2	0	15	0	1	0	1	-.085	5	-3.337	1
553		max	925.563	3	153.416	2	124.236	4	0	1	0	1	2.054	3
554		min	-506.194	2	-1.608	6	0	1	0	1	-.02	5	-3.379	1
555		max	938.054	3	1076.415	3	166.996	4	0	1	0	1	1.804	3
556		min	-396.934	2	-1983.724	1	0	1	0	4	-.229	4	-3.012	1
557		max	938.332	3	1075.378	3	168.237	4	0	1	0	1	1.236	3
558		min	-396.563	2	-1985.107	1	0	1	0	4	-.141	4	-1.965	1
559		max	938.61	3	1074.34	3	169.479	4	0	1	0	1	.669	3
560		min	-396.192	2	-1986.491	1	0	1	0	4	-.052	4	-.917	1
561		max	938.888	3	1073.302	3	170.72	4	0	1	.038	4	.207	2
562	M8	min	-395.822	2	-1987.875	1	0	1	0	4	0	1	-.004	13
563		max	939.166	3	1072.264	3	171.962	4	0	1	.128	4	1.232	2
564		min	-395.451	2	-1989.258	1	0	1	0	4	0	1	-.464	3
565		max	939.444	3	1071.227	3	173.203	4	0	1	.22	4	2.258	2
566		min	-395.08	2	-1990.642	1	0	1	0	4	0	1	-1.029	3
567		max	-8.826	12	2165.743	2	0	1	0	4	.218	4	1.164	2
568		min	-287.703	1	-978.674	3	-39.641	5	0	1	0	1	-.538	3
569		max	-8.641	12	2164.359	2	0	1	0	4	.198	4	.026	1
570		min	-287.333	1	-979.712	3	-38.4	5	0	1	0	1	-.021	3
571		max	132.703	1	693.641	3	79.503	1	0	3	-.008	12	0	3
572	M9	min	4.82	12	-534.357	1	3.549	12	0	1	-.189	1	-.015	1
573		max	133.074	1	692.603	3	80.48	4	0	3	-.007	12	.268	1
574		min	5.006	12	-535.741	1	3.549	12	0	1	-.147	1	-.365	3
575		max	283.888	3	601.016	1	78.569	1	0	1	-.005	12	.537	1
576		min	-174.97	2	-509.501	3	-9.324	5	0	3	-.105	1	-.716	3
577		max	284.166	3	599.632	1	78.569	1	0	1	-.003	12	.22	1
578		min	-174.599	2	-510.539	3	-8.082	5	0	3	-.064	1	-.447	3
579		max	284.444	3	598.249	1	78.569	1	0	1	-.001	12	-.004	15
580		min	-174.229	2	-511.576	3	-6.841	5	0	3	-.023	4	-.177	3
581		max	284.722	3	596.865	1	78.569	1	0	1	.019	1	.093	3
582	M10	min	-173.858	2	-512.614	3	-5.599	5	0	3	-.021	5	-.411	1
583		max	285.001	3	595.481	1	78.569	1	0	1	.061	1	.364	3
584		min	-173.487	2	-513.652	3	-4.358	5	0	3	-.024	5	-.726	1
585		max	285.279	3	594.098	1	78.569	1	0	1	.102	1	.635	3
586		min	-173.116	2	-514.689	3	-3.116	5	0	3	-.026	5	-1.04	1
587		max	293.032	3	46.754	2	116.272	1	0	3	-.003	12	.741	3
588		min	-120.236	2	.422	15	5.017	12	0	9	-.133	4	-1.185	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	293.31	3	45.371	2	116.272	1	0	3	0	1	.723	3
590		min	-119.866	2	.005	15	5.017	12	0	9	-.093	4	-1.198	1
591	11	max	293.588	3	43.987	2	116.272	1	0	3	.062	1	.705	3
592		min	-119.495	2	-1.697	6	5.017	12	0	9	-.066	5	-1.21	1
593	12	max	301.283	3	342.769	3	147.594	4	0	3	-.004	12	.614	3
594		min	-75.274	10	-643.523	1	3.204	12	0	2	-.197	4	-1.069	1
595	13	max	301.561	3	341.731	3	148.836	4	0	3	-.003	12	.434	3
596		min	-74.965	10	-644.907	1	3.204	12	0	2	-.119	4	-.729	1
597	14	max	301.839	3	340.693	3	150.077	4	0	3	0	12	.254	3
598		min	-74.656	10	-646.291	1	3.204	12	0	2	-.04	4	-.388	1
599	15	max	302.117	3	339.656	3	151.319	4	0	3	.039	4	.074	3
600		min	-74.347	10	-647.674	1	3.204	12	0	2	0	12	-.047	1
601	16	max	302.395	3	338.618	3	152.56	4	0	3	.119	4	.317	2
602		min	-74.038	10	-649.058	1	3.204	12	0	2	.003	12	-.105	3
603	17	max	302.673	3	337.58	3	153.802	4	0	3	.2	4	.652	2
604		min	-73.729	10	-650.441	1	3.204	12	0	2	.004	12	-.283	3
605	18	max	-4.88	12	642.354	2	84.116	1	0	2	.186	4	.328	2
606		min	-133.263	1	-285.277	3	-77.99	5	0	3	.006	12	-.14	3
607	19	max	-4.695	12	640.97	2	84.116	1	0	2	.19	1	.011	3
608		min	-132.892	1	-286.315	3	-76.748	5	0	3	.008	12	-.013	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.122	1	.005	3	9.678e-3	1	NC	1	NC	1
2			min	-.456	4	-.025	3	-.002	2	-1.866e-3	3	NC	1	NC	1
3		2	max	0	1	.278	3	.029	1	1.108e-2	1	NC	5	NC	2
4			min	-.456	4	-.089	1	-.012	5	-1.944e-3	3	773.511	3	8439.325	1
5		3	max	0	1	.523	3	.069	1	1.249e-2	1	NC	5	NC	3
6			min	-.456	4	-.256	1	-.014	5	-2.021e-3	3	427.67	3	3463.596	1
7		4	max	0	1	.67	3	.103	1	1.39e-2	1	NC	5	NC	3
8			min	-.456	4	-.348	1	-.01	5	-2.099e-3	3	336.675	3	2295.468	1
9		5	max	0	1	.703	3	.121	1	1.53e-2	1	NC	5	NC	3
10			min	-.456	4	-.352	1	-.002	5	-2.177e-3	3	321.411	3	1957.508	1
11		6	max	0	1	.624	3	.117	1	1.671e-2	1	NC	5	NC	3
12			min	-.456	4	-.271	1	.004	15	-2.254e-3	3	360.672	3	2030.182	1
13		7	max	0	1	.456	3	.092	1	1.812e-2	1	NC	5	NC	3
14			min	-.456	4	-.124	1	.003	10	-2.332e-3	3	486.843	3	2593.271	1
15		8	max	0	1	.243	3	.053	1	1.952e-2	1	NC	4	NC	2
16			min	-.456	4	.001	15	-.001	10	-2.409e-3	3	874.038	3	4515.723	1
17		9	max	0	1	.217	2	.016	3	2.093e-2	1	NC	4	NC	1
18			min	-.456	4	.005	15	-.005	10	-2.487e-3	3	2359.309	2	NC	1
19		10	max	0	1	.284	1	.016	3	2.234e-2	1	NC	3	NC	1
20			min	-.456	4	-.037	3	-.01	2	-2.564e-3	3	1444.191	1	NC	1
21		11	max	0	12	.217	2	.016	3	2.093e-2	1	NC	4	NC	1
22			min	-.456	4	.005	15	-.01	5	-2.487e-3	3	2359.309	2	NC	1
23		12	max	0	12	.243	3	.053	1	1.952e-2	1	NC	4	NC	2
24			min	-.456	4	.001	15	-.01	5	-2.409e-3	3	874.038	3	4515.723	1
25		13	max	0	12	.456	3	.092	1	1.812e-2	1	NC	5	NC	3
26			min	-.456	4	-.124	1	-.003	5	-2.332e-3	3	486.843	3	2593.271	1
27		14	max	0	12	.624	3	.117	1	1.671e-2	1	NC	5	NC	3
28			min	-.456	4	-.271	1	.004	15	-2.254e-3	3	360.672	3	2030.182	1
29		15	max	0	12	.703	3	.121	1	1.53e-2	1	NC	5	NC	3
30			min	-.456	4	-.352	1	.007	10	-2.177e-3	3	321.411	3	1957.508	1
31		16	max	0	12	.67	3	.103	1	1.39e-2	1	NC	5	NC	3
32			min	-.456	4	-.348	1	.006	10	-2.099e-3	3	336.675	3	2295.468	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.523	3	.069	1	1.249e-2	1	NC	5	NC	3
34		min	-456	4	-256	1	.004	10	-2.021e-3	3	427.67	3	3463.596	1
35	18	max	0	12	.278	3	.029	1	1.108e-2	1	NC	5	NC	2
36		min	-456	4	-.089	1	0	10	-1.944e-3	3	773.511	3	8439.325	1
37	19	max	0	12	.122	1	.005	3	9.678e-3	1	NC	1	NC	1
38		min	-456	4	-.025	3	-.002	2	-1.866e-3	3	NC	1	NC	1
39	M14	1	max	0	.22	3	.005	3	5.991e-3	1	NC	1	NC	1
40		min	-.366	4	-.389	1	-.002	2	-3.95e-3	3	NC	1	NC	1
41	2	max	0	1	.522	3	.02	1	7.177e-3	1	NC	5	NC	1
42		min	-.366	4	-.738	1	-.017	5	-4.798e-3	3	671.183	1	NC	1
43	3	max	0	1	.777	3	.055	1	8.364e-3	1	NC	5	NC	2
44		min	-.366	4	-1.038	1	-.021	5	-5.646e-3	3	360.399	1	4339.711	1
45	4	max	0	1	.956	3	.088	1	9.551e-3	1	NC	15	NC	3
46		min	-.366	4	-1.258	1	-.014	5	-6.493e-3	3	269.342	1	2690.102	1
47	5	max	0	1	1.04	3	.107	1	1.074e-2	1	NC	15	NC	3
48		min	-.366	4	-1.379	1	-.002	5	-7.341e-3	3	236.313	1	2211.589	1
49	6	max	0	1	1.032	3	.106	1	1.192e-2	1	NC	15	NC	3
50		min	-.366	4	-1.402	1	.005	10	-8.188e-3	3	231.038	1	2240.644	1
51	7	max	0	1	.947	3	.085	1	1.311e-2	1	NC	15	NC	3
52		min	-.366	4	-1.341	1	.003	10	-9.036e-3	3	245.778	1	2814.174	1
53	8	max	0	1	.817	3	.05	1	1.43e-2	1	NC	15	NC	2
54		min	-.366	4	-1.229	1	0	10	-9.884e-3	3	278.685	1	4828.185	1
55	9	max	0	1	.69	3	.024	4	1.548e-2	1	NC	15	NC	1
56		min	-.366	4	-1.112	1	-.004	10	-1.073e-2	3	323.46	1	9778.119	4
57	10	max	0	1	.631	3	.014	3	1.667e-2	1	NC	5	NC	1
58		min	-.366	4	-1.056	1	-.009	2	-1.158e-2	3	350.58	1	NC	1
59	11	max	0	12	.69	3	.015	3	1.548e-2	1	NC	15	NC	1
60		min	-.366	4	-1.112	1	-.017	5	-1.073e-2	3	323.46	1	NC	1
61	12	max	0	12	.817	3	.05	1	1.43e-2	1	NC	15	NC	2
62		min	-.366	4	-1.229	1	-.02	5	-9.884e-3	3	278.685	1	4828.185	1
63	13	max	0	12	.947	3	.085	1	1.311e-2	1	NC	15	NC	3
64		min	-.366	4	-1.341	1	-.013	5	-9.036e-3	3	245.778	1	2814.174	1
65	14	max	0	12	1.032	3	.106	1	1.192e-2	1	NC	15	NC	3
66		min	-.366	4	-1.402	1	0	15	-8.188e-3	3	231.038	1	2240.644	1
67	15	max	0	12	1.04	3	.107	1	1.074e-2	1	NC	15	NC	3
68		min	-.366	4	-1.379	1	.006	10	-7.341e-3	3	236.313	1	2211.589	1
69	16	max	0	12	.956	3	.088	1	9.551e-3	1	NC	15	NC	3
70		min	-.366	4	-1.258	1	.005	10	-6.493e-3	3	269.342	1	2690.102	1
71	17	max	0	12	.777	3	.055	1	8.364e-3	1	NC	5	NC	2
72		min	-.366	4	-1.038	1	.003	10	-5.646e-3	3	360.399	1	4339.711	1
73	18	max	0	12	.522	3	.025	4	7.177e-3	1	NC	5	NC	1
74		min	-.366	4	-.738	1	0	10	-4.798e-3	3	671.183	1	9489.459	4
75	19	max	0	12	.22	3	.005	3	5.991e-3	1	NC	1	NC	1
76		min	-.366	4	-.389	1	-.002	2	-3.95e-3	3	NC	1	NC	1
77	M15	1	max	0	.225	3	.004	3	3.352e-3	3	NC	1	NC	1
78		min	-.309	4	-.389	1	-.002	2	-6.106e-3	1	NC	1	NC	1
79	2	max	0	12	.422	3	.02	1	4.073e-3	3	NC	5	NC	1
80		min	-.309	4	-.778	1	-.027	5	-7.319e-3	1	601.02	1	8484.34	5
81	3	max	0	12	.593	3	.056	1	4.793e-3	3	NC	5	NC	2
82		min	-.309	4	-1.112	1	-.033	5	-8.533e-3	1	323.687	1	4326.828	1
83	4	max	0	12	.721	3	.089	1	5.514e-3	3	NC	15	NC	3
84		min	-.309	4	-1.351	1	-.024	5	-9.747e-3	1	243.144	1	2683.53	1
85	5	max	0	12	.794	3	.107	1	6.235e-3	3	NC	15	NC	3
86		min	-.31	4	-1.477	1	-.006	5	-1.096e-2	1	214.999	1	2206.366	1
87	6	max	0	12	.814	3	.106	1	6.955e-3	3	NC	15	NC	3
88		min	-.31	4	-1.489	1	.006	10	-1.217e-2	1	212.632	1	2234.752	1
89	7	max	0	12	.788	3	.085	1	7.676e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-.31	4	-1.406	1	.003	10	-1.339e-2	1	230.015	1	2804.387	1
91		8	max	0	12	.732	3	.05	1	8.396e-3	3	NC	15	NC	2
92			min	-.31	4	-1.265	1	0	10	-1.46e-2	1	266.986	1	4798.714	1
93		9	max	0	12	.673	3	.032	4	9.117e-3	3	NC	15	NC	1
94			min	-.31	4	-1.123	1	-.004	10	-1.582e-2	1	318.614	1	7317.374	4
95		10	max	0	1	.644	3	.013	3	9.837e-3	3	NC	5	NC	1
96			min	-.31	4	-1.055	1	-.008	2	-1.703e-2	1	350.929	1	NC	1
97		11	max	0	1	.673	3	.015	1	9.117e-3	3	NC	15	NC	1
98			min	-.31	4	-1.123	1	-.025	5	-1.582e-2	1	318.614	1	9156.188	5
99		12	max	0	1	.732	3	.05	1	8.396e-3	3	NC	15	NC	2
100			min	-.31	4	-1.265	1	-.03	5	-1.46e-2	1	266.986	1	4798.714	1
101		13	max	0	1	.788	3	.085	1	7.676e-3	3	NC	15	NC	3
102			min	-.309	4	-1.406	1	-.02	5	-1.339e-2	1	230.015	1	2804.387	1
103		14	max	0	1	.814	3	.106	1	6.955e-3	3	NC	15	NC	3
104			min	-.309	4	-1.489	1	-.002	5	-1.217e-2	1	212.632	1	2234.752	1
105		15	max	0	1	.794	3	.107	1	6.235e-3	3	NC	15	NC	3
106			min	-.309	4	-1.477	1	.006	10	-1.096e-2	1	214.999	1	2206.366	1
107		16	max	0	1	.721	3	.089	1	5.514e-3	3	NC	15	NC	3
108			min	-.309	4	-1.351	1	.005	10	-9.747e-3	1	243.144	1	2683.53	1
109		17	max	0	1	.593	3	.056	1	4.793e-3	3	NC	5	NC	2
110			min	-.309	4	-1.112	1	.003	10	-8.533e-3	1	323.687	1	4326.828	1
111		18	max	0	1	.422	3	.034	4	4.073e-3	3	NC	5	NC	1
112			min	-.309	4	-.778	1	0	10	-7.319e-3	1	601.02	1	6906.628	4
113		19	max	0	1	.225	3	.004	3	3.352e-3	3	NC	1	NC	1
114			min	-.309	4	-.389	1	-.002	2	-6.106e-3	1	NC	1	NC	1
115	M16	1	max	0	12	.116	1	.004	3	5.883e-3	3	NC	1	NC	1
116			min	-.138	4	-.075	3	-.002	2	-8.935e-3	1	NC	1	NC	1
117		2	max	0	12	.033	3	.029	1	6.89e-3	3	NC	5	NC	2
118			min	-.138	4	-.159	2	-.02	5	-1.016e-2	1	893.337	2	8483.2	1
119		3	max	0	12	.118	3	.069	1	7.897e-3	3	NC	5	NC	3
120			min	-.138	4	-.367	2	-.025	5	-1.139e-2	1	497.334	2	3469.892	1
121		4	max	0	12	.162	3	.103	1	8.905e-3	3	NC	5	NC	3
122			min	-.138	4	-.487	2	-.019	5	-1.262e-2	1	396.589	2	2294.948	1
123		5	max	0	12	.16	3	.121	1	9.912e-3	3	NC	5	NC	3
124			min	-.138	4	-.501	2	-.007	5	-1.385e-2	1	387.336	2	1953.351	1
125		6	max	0	12	.113	3	.117	1	1.092e-2	3	NC	5	NC	3
126			min	-.138	4	-.413	2	.004	15	-1.508e-2	1	453.739	2	2020.857	1
127		7	max	0	12	.03	3	.092	1	1.193e-2	3	NC	5	NC	3
128			min	-.138	4	-.244	2	.004	10	-1.631e-2	1	674.313	2	2570.023	1
129		8	max	0	12	.017	9	.054	1	1.293e-2	3	NC	3	NC	2
130			min	-.138	4	-.068	3	0	10	-1.754e-2	1	1676.695	2	4424.512	1
131		9	max	0	12	.184	1	.022	4	1.394e-2	3	NC	4	NC	1
132			min	-.138	4	-.155	3	-.003	10	-1.877e-2	1	2926.398	3	NC	1
133		10	max	0	1	.267	1	.012	3	1.495e-2	3	NC	5	NC	1
134			min	-.138	4	-.193	3	-.008	2	-2.e-2	1	1553.824	1	NC	1
135		11	max	0	1	.184	1	.016	1	1.394e-2	3	NC	4	NC	1
136			min	-.138	4	-.155	3	-.016	5	-1.877e-2	1	2926.398	3	NC	1
137		12	max	0	1	.017	9	.054	1	1.293e-2	3	NC	3	NC	2
138			min	-.138	4	-.068	3	-.017	5	-1.754e-2	1	1676.695	2	4424.512	1
139		13	max	0	1	.03	3	.092	1	1.193e-2	3	NC	5	NC	3
140			min	-.138	4	-.244	2	-.008	5	-1.631e-2	1	674.313	2	2570.023	1
141		14	max	0	1	.113	3	.117	1	1.092e-2	3	NC	5	NC	3
142			min	-.138	4	-.413	2	.004	15	-1.508e-2	1	453.739	2	2020.857	1
143		15	max	0	1	.16	3	.121	1	9.912e-3	3	NC	5	NC	3
144			min	-.138	4	-.501	2	.008	10	-1.385e-2	1	387.336	2	1953.351	1
145		16	max	0	1	.162	3	.103	1	8.905e-3	3	NC	5	NC	3
146			min	-.138	4	-.487	2	.007	10	-1.262e-2	1	396.589	2	2294.948	1



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Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.118	3	.069	1	7.897e-3	3	NC	5	NC	3
148			min	-1.138	4	-.367	2	.004	10	-1.139e-2	1	497.334	2	3469.892	1
149		18	max	0	1	.033	3	.029	4	6.89e-3	3	NC	5	NC	2
150			min	-1.138	4	-.159	2	0	10	-1.016e-2	1	893.337	2	7902.067	4
151		19	max	0	1	.116	1	.004	3	5.883e-3	3	NC	1	NC	1
152			min	-1.138	4	-.075	3	-.002	2	-8.935e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.003	2	.006	1	1.181e-3	5	NC	1	NC	2
154			min	-.006	3	-.007	3	-.432	4	-1.559e-4	1	NC	1	110.698	4
155		2	max	.005	1	.003	2	.006	1	1.262e-3	5	NC	1	NC	2
156			min	-.005	3	-.006	3	-.397	4	-1.445e-4	1	NC	1	120.618	4
157		3	max	.005	1	.002	2	.005	1	1.342e-3	5	NC	1	NC	2
158			min	-.005	3	-.006	3	-.361	4	-1.33e-4	1	NC	1	132.413	4
159		4	max	.005	1	.002	2	.005	1	1.422e-3	5	NC	1	NC	1
160			min	-.005	3	-.006	3	-.326	4	-1.216e-4	1	NC	1	146.576	4
161		5	max	.004	1	.001	2	.004	1	1.503e-3	5	NC	1	NC	1
162			min	-.004	3	-.006	3	-.292	4	-1.102e-4	1	NC	1	163.773	4
163		6	max	.004	1	.001	2	.004	1	1.583e-3	5	NC	1	NC	1
164			min	-.004	3	-.005	3	-.259	4	-9.872e-5	1	NC	1	184.934	4
165		7	max	.004	1	0	2	.003	1	1.663e-3	5	NC	1	NC	1
166			min	-.004	3	-.005	3	-.226	4	-8.728e-5	1	NC	1	211.376	4
167		8	max	.003	1	0	2	.003	1	1.744e-3	4	NC	1	NC	1
168			min	-.003	3	-.005	3	-.195	4	-7.584e-5	1	NC	1	245.027	4
169		9	max	.003	1	0	2	.002	1	1.828e-3	4	NC	1	NC	1
170			min	-.003	3	-.004	3	-.166	4	-6.441e-5	1	NC	1	288.792	4
171		10	max	.003	1	0	15	.002	1	1.913e-3	4	NC	1	NC	1
172			min	-.003	3	-.004	3	-.138	4	-5.297e-5	1	NC	1	347.225	4
173		11	max	.002	1	0	15	.002	1	1.997e-3	4	NC	1	NC	1
174			min	-.003	3	-.004	3	-.112	4	-4.153e-5	1	NC	1	427.797	4
175		12	max	.002	1	0	15	.001	1	2.081e-3	4	NC	1	NC	1
176			min	-.002	3	-.003	3	-.088	4	-3.009e-5	1	NC	1	543.477	4
177		13	max	.002	1	0	15	0	1	2.166e-3	4	NC	1	NC	1
178			min	-.002	3	-.003	3	-.067	4	-1.865e-5	1	NC	1	718.444	4
179		14	max	.002	1	0	15	0	1	2.25e-3	4	NC	1	NC	1
180			min	-.002	3	-.003	3	-.048	4	-7.211e-6	1	NC	1	1002.177	4
181		15	max	.001	1	0	15	0	1	2.335e-3	4	NC	1	NC	1
182			min	-.001	3	-.002	3	-.032	4	-2.03e-7	3	NC	1	1509.648	4
183		16	max	0	1	0	15	0	1	2.419e-3	4	NC	1	NC	1
184			min	0	3	-.002	3	-.019	4	3.965e-7	12	NC	1	2562.389	4
185		17	max	0	1	0	15	0	1	2.503e-3	4	NC	1	NC	1
186			min	0	3	-.001	3	-.009	4	9.12e-7	12	NC	1	5375.331	4
187		18	max	0	1	0	15	0	1	2.588e-3	4	NC	1	NC	1
188			min	0	3	0	3	-.003	4	1.427e-6	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.672e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.943e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-6.127e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-5.962e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	3.988e-6	1	NC	1	NC	1
194			min	0	2	-.001	6	0	12	0	15	NC	1	NC	1
195		3	max	0	3	0	15	.025	4	6.01e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	9.607e-7	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.037	4	1.2e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	1.747e-6	12	NC	1	NC	1
199		5	max	0	3	-.001	15	.049	4	1.798e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	2.534e-6	12	NC	1	9657.556	4
201		6	max	.001	3	-.002	15	.059	4	2.397e-3	4	NC	1	NC	1
202			min	0	2	-.008	6	0	12	3.321e-6	12	NC	1	8738.26	5
203		7	max	.001	3	-.002	15	.07	4	2.995e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204		min	-.001	2	-.01	6	0	12	4.107e-6	12	9464.738	6	8344.438	5
205	8	max	.002	3	-.002	15	.079	4	3.594e-3	4	NC	1	NC	1
206		min	-.001	2	-.011	6	0	12	4.894e-6	12	8424.121	6	8331.433	5
207	9	max	.002	3	-.003	15	.089	4	4.193e-3	4	NC	1	NC	1
208		min	-.001	2	-.012	6	0	12	5.681e-6	12	7801.155	6	8658.614	5
209	10	max	.002	3	-.003	15	.098	4	4.791e-3	4	NC	2	NC	1
210		min	-.002	2	-.012	6	0	12	6.467e-6	12	7484.601	6	9359.842	5
211	11	max	.002	3	-.003	15	.106	4	5.39e-3	4	NC	2	NC	1
212		min	-.002	2	-.012	6	0	12	7.254e-6	12	7426.784	6	NC	1
213	12	max	.003	3	-.003	15	.115	4	5.988e-3	4	NC	1	NC	1
214		min	-.002	2	-.012	6	0	12	8.041e-6	12	7625.123	6	NC	1
215	13	max	.003	3	-.002	15	.123	4	6.587e-3	4	NC	1	NC	1
216		min	-.002	2	-.011	6	0	12	8.827e-6	12	8123.36	6	NC	1
217	14	max	.003	3	-.002	15	.131	4	7.185e-3	4	NC	1	NC	1
218		min	-.002	2	-.01	6	0	12	9.614e-6	12	9035.019	6	NC	1
219	15	max	.003	3	-.002	15	.14	4	7.784e-3	4	NC	1	NC	1
220		min	-.002	2	-.009	1	0	12	1.04e-5	12	NC	1	NC	1
221	16	max	.003	3	-.001	15	.148	4	8.383e-3	4	NC	1	NC	1
222		min	-.003	2	-.008	1	0	12	1.119e-5	12	NC	1	NC	1
223	17	max	.004	3	0	15	.157	4	8.981e-3	4	NC	1	NC	1
224		min	-.003	2	-.006	1	0	12	1.197e-5	12	NC	1	NC	1
225	18	max	.004	3	0	15	.166	4	9.58e-3	4	NC	1	NC	1
226		min	-.003	2	-.005	1	0	12	1.276e-5	12	NC	1	NC	1
227	19	max	.004	3	0	5	.176	4	1.018e-2	4	NC	1	NC	1
228		min	-.003	2	-.003	1	0	12	1.355e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.002	2	12	-2.611e-7	12	NC	1	NC	2
230		min	0	3	-.004	3	-.176	4	-8.331e-4	4	NC	1	140.986	4
231	2	max	.003	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
232		min	0	3	-.004	3	-.162	4	-8.331e-4	4	NC	1	153.492	4
233	3	max	.003	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
234		min	0	3	-.004	3	-.147	4	-8.331e-4	4	NC	1	168.364	4
235	4	max	.003	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
236		min	0	3	-.003	3	-.133	4	-8.331e-4	4	NC	1	186.219	4
237	5	max	.002	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
238		min	0	3	-.003	3	-.119	4	-8.331e-4	4	NC	1	207.896	4
239	6	max	.002	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
240		min	0	3	-.003	3	-.106	4	-8.331e-4	4	NC	1	234.555	4
241	7	max	.002	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
242		min	0	3	-.003	3	-.093	4	-8.331e-4	4	NC	1	267.847	4
243	8	max	.002	1	.002	2	0	12	-2.611e-7	12	NC	1	NC	2
244		min	0	3	-.003	3	-.08	4	-8.331e-4	4	NC	1	310.177	4
245	9	max	.002	1	.001	2	0	12	-2.611e-7	12	NC	1	NC	1
246		min	0	3	-.002	3	-.068	4	-8.331e-4	4	NC	1	365.167	4
247	10	max	.002	1	.001	2	0	12	-2.611e-7	12	NC	1	NC	1
248		min	0	3	-.002	3	-.057	4	-8.331e-4	4	NC	1	438.48	4
249	11	max	.001	1	.001	2	0	12	-2.611e-7	12	NC	1	NC	1
250		min	0	3	-.002	3	-.046	4	-8.331e-4	4	NC	1	539.381	4
251	12	max	.001	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
252		min	0	3	-.002	3	-.036	4	-8.331e-4	4	NC	1	683.898	4
253	13	max	.001	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
254		min	0	3	-.001	3	-.028	4	-8.331e-4	4	NC	1	901.787	4
255	14	max	0	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
256		min	0	3	-.001	3	-.02	4	-8.331e-4	4	NC	1	1253.595	4
257	15	max	0	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
258		min	0	3	0	3	-.013	4	-8.331e-4	4	NC	1	1878.918	4
259	16	max	0	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
260		min	0	3	0	3	-.008	4	-8.331e-4	4	NC	1	3163.759	4



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-8.331e-4	4	NC	1	6539.852	4
263		18	max	0	1	0	2	0	12	-2.611e-7	12	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-8.331e-4	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	-2.611e-7	12	NC	1	NC	1
266			min	0	1	0	1	0	1	-8.331e-4	4	NC	1	NC	1
267	M6	1	max	.017	1	.014	2	0	1	1.232e-3	4	NC	3	NC	1
268			min	-.019	3	-.021	3	-.436	4	0	1	3449.392	2	109.773	4
269		2	max	.016	1	.013	2	0	1	1.311e-3	4	NC	3	NC	1
270			min	-.017	3	-.019	3	-.4	4	0	1	3772.583	2	119.611	4
271		3	max	.015	1	.012	2	0	1	1.39e-3	4	NC	3	NC	1
272			min	-.016	3	-.018	3	-.364	4	0	1	4159.743	2	131.309	4
273		4	max	.014	1	.01	2	0	1	1.469e-3	4	NC	3	NC	1
274			min	-.015	3	-.017	3	-.329	4	0	1	4628.394	2	145.355	4
275		5	max	.013	1	.009	2	0	1	1.548e-3	4	NC	1	NC	1
276			min	-.014	3	-.016	3	-.295	4	0	1	5202.683	2	162.411	4
277		6	max	.013	1	.008	2	0	1	1.627e-3	4	NC	1	NC	1
278			min	-.013	3	-.015	3	-.261	4	0	1	5916.637	2	183.397	4
279		7	max	.012	1	.007	2	0	1	1.706e-3	4	NC	1	NC	1
280			min	-.012	3	-.014	3	-.228	4	0	1	6819.466	2	209.623	4
281		8	max	.011	1	.006	2	0	1	1.785e-3	4	NC	1	NC	1
282			min	-.011	3	-.013	3	-.197	4	0	1	7984.518	2	242.999	4
283		9	max	.01	1	.005	2	0	1	1.864e-3	4	NC	1	NC	1
284			min	-.01	3	-.011	3	-.167	4	0	1	9525.108	2	286.407	4
285		10	max	.009	1	.004	2	0	1	1.943e-3	4	NC	1	NC	1
286			min	-.009	3	-.01	3	-.139	4	0	1	NC	1	344.365	4
287		11	max	.008	1	.003	2	0	1	2.022e-3	4	NC	1	NC	1
288			min	-.008	3	-.009	3	-.113	4	0	1	NC	1	424.286	4
289		12	max	.007	1	.003	2	0	1	2.101e-3	4	NC	1	NC	1
290			min	-.007	3	-.008	3	-.089	4	0	1	NC	1	539.035	4
291		13	max	.006	1	.002	2	0	1	2.18e-3	4	NC	1	NC	1
292			min	-.006	3	-.007	3	-.067	4	0	1	NC	1	712.604	4
293		14	max	.005	1	.001	2	0	1	2.259e-3	4	NC	1	NC	1
294			min	-.005	3	-.006	3	-.048	4	0	1	NC	1	994.093	4
295		15	max	.004	1	0	2	0	1	2.338e-3	4	NC	1	NC	1
296			min	-.004	3	-.005	3	-.032	4	0	1	NC	1	1497.603	4
297		16	max	.003	1	0	2	0	1	2.417e-3	4	NC	1	NC	1
298			min	-.003	3	-.003	3	-.019	4	0	1	NC	1	2542.299	4
299		17	max	.002	1	0	2	0	1	2.496e-3	4	NC	1	NC	1
300			min	-.002	3	-.002	3	-.009	4	0	1	NC	1	5334.55	4
301		18	max	0	1	0	2	0	1	2.575e-3	4	NC	1	NC	1
302			min	-.001	3	-.001	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.654e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.901e-4	4	NC	1	NC	1
307		2	max	0	3	0	15	.013	4	0	1	NC	1	NC	1
308			min	0	2	-.002	3	0	1	-4.382e-6	5	NC	1	NC	1
309		3	max	.001	3	0	15	.025	4	5.836e-4	4	NC	1	NC	1
310			min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
311		4	max	.002	3	-.001	15	.037	4	1.171e-3	4	NC	1	NC	1
312			min	-.002	2	-.006	3	0	1	0	1	NC	1	NC	1
313		5	max	.003	3	-.002	15	.048	4	1.757e-3	4	NC	1	NC	1
314			min	-.003	2	-.008	3	0	1	0	1	NC	1	9231.684	4
315		6	max	.004	3	-.002	15	.059	4	2.344e-3	4	NC	1	NC	1
316			min	-.003	2	-.009	3	0	1	0	1	9880.386	3	8322.485	4
317		7	max	.004	3	-.002	15	.069	4	2.931e-3	4	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.004	2	-.011	3	0	1	0	1	8767.433	3	7921.604	4
319	8	max	.005	3	-.003	15	.079	4	3.518e-3	4	NC	1	NC	1
320		min	-.005	2	-.011	3	0	1	0	1	8100.192	3	7877.943	4
321	9	max	.006	3	-.003	15	.088	4	4.105e-3	4	NC	1	NC	1
322		min	-.005	2	-.012	4	0	1	0	1	7742.137	3	8147.208	4
323	10	max	.007	3	-.003	15	.096	4	4.692e-3	4	NC	1	NC	1
324		min	-.006	2	-.013	4	0	1	0	1	7537.195	4	8752.626	4
325	11	max	.007	3	-.003	15	.105	4	5.279e-3	4	NC	1	NC	1
326		min	-.007	2	-.013	4	0	1	0	1	7476.015	4	9786.585	4
327	12	max	.008	3	-.003	15	.113	4	5.865e-3	4	NC	1	NC	1
328		min	-.007	2	-.013	4	0	1	0	1	7673.136	4	NC	1
329	13	max	.009	3	-.003	15	.121	4	6.452e-3	4	NC	1	NC	1
330		min	-.008	2	-.012	4	0	1	0	1	8172.262	4	NC	1
331	14	max	.01	3	-.003	15	.129	4	7.039e-3	4	NC	1	NC	1
332		min	-.009	2	-.012	1	0	1	0	1	9087.345	4	NC	1
333	15	max	.01	3	-.002	15	.137	4	7.626e-3	4	NC	1	NC	1
334		min	-.01	2	-.011	1	0	1	0	1	NC	1	NC	1
335	16	max	.011	3	-.002	15	.145	4	8.213e-3	4	NC	1	NC	1
336		min	-.01	2	-.01	1	0	1	0	1	NC	1	NC	1
337	17	max	.012	3	-.001	15	.154	4	8.8e-3	4	NC	1	NC	1
338		min	-.011	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.012	3	0	15	.163	4	9.387e-3	4	NC	1	NC	1
340		min	-.012	2	-.009	1	0	1	0	1	NC	1	NC	1
341	19	max	.013	3	0	15	.172	4	9.974e-3	4	NC	1	NC	1
342		min	-.012	2	-.008	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.01	2	0	0	1	NC	1	NC	1
344		min	-.003	3	-.013	3	-.172	4	-8.556e-4	4	NC	1	144.068	4
345	2	max	.009	1	.01	2	0	1	0	1	NC	1	NC	1
346		min	-.003	3	-.012	3	-.158	4	-8.556e-4	4	NC	1	156.849	4
347	3	max	.008	1	.009	2	0	1	0	1	NC	1	NC	1
348		min	-.003	3	-.011	3	-.144	4	-8.556e-4	4	NC	1	172.049	4
349	4	max	.008	1	.009	2	0	1	0	1	NC	1	NC	1
350		min	-.003	3	-.011	3	-.13	4	-8.556e-4	4	NC	1	190.297	4
351	5	max	.007	1	.008	2	0	1	0	1	NC	1	NC	1
352		min	-.002	3	-.01	3	-.117	4	-8.556e-4	4	NC	1	212.451	4
353	6	max	.007	1	.007	2	0	1	0	1	NC	1	NC	1
354		min	-.002	3	-.009	3	-.103	4	-8.556e-4	4	NC	1	239.697	4
355	7	max	.006	1	.007	2	0	1	0	1	NC	1	NC	1
356		min	-.002	3	-.009	3	-.091	4	-8.556e-4	4	NC	1	273.721	4
357	8	max	.006	1	.006	2	0	1	0	1	NC	1	NC	1
358		min	-.002	3	-.008	3	-.078	4	-8.556e-4	4	NC	1	316.983	4
359	9	max	.005	1	.006	2	0	1	0	1	NC	1	NC	1
360		min	-.002	3	-.007	3	-.066	4	-8.556e-4	4	NC	1	373.183	4
361	10	max	.005	1	.005	2	0	1	0	1	NC	1	NC	1
362		min	-.002	3	-.006	3	-.055	4	-8.556e-4	4	NC	1	448.109	4
363	11	max	.004	1	.005	2	0	1	0	1	NC	1	NC	1
364		min	-.001	3	-.006	3	-.045	4	-8.556e-4	4	NC	1	551.231	4
365	12	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
366		min	-.001	3	-.005	3	-.035	4	-8.556e-4	4	NC	1	698.929	4
367	13	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
368		min	-.001	3	-.004	3	-.027	4	-8.556e-4	4	NC	1	921.614	4
369	14	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.004	3	-.019	4	-8.556e-4	4	NC	1	1281.166	4
371	15	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.003	3	-.013	4	-8.556e-4	4	NC	1	1920.256	4
373	16	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.008	4	-8.556e-4	4	NC	1	3233.392	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.001	3	-.004	4	-8.556e-4	4	NC	1	6683.856	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-8.556e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-8.556e-4	4	NC	1	NC	1
381	M10	1	max	.005	1	.003	2	0	12	1.231e-3	4	NC	1	NC	2
382			min	-.006	3	-.007	3	-.435	4	7.335e-6	12	NC	1	109.916	4
383		2	max	.005	1	.003	2	0	12	1.31e-3	4	NC	1	NC	2
384			min	-.005	3	-.006	3	-.4	4	6.82e-6	12	NC	1	119.767	4
385		3	max	.005	1	.002	2	0	12	1.389e-3	4	NC	1	NC	2
386			min	-.005	3	-.006	3	-.364	4	6.304e-6	12	NC	1	131.48	4
387		4	max	.005	1	.002	2	0	12	1.467e-3	4	NC	1	NC	1
388			min	-.005	3	-.006	3	-.329	4	5.789e-6	12	NC	1	145.544	4
389		5	max	.004	1	.001	2	0	12	1.546e-3	4	NC	1	NC	1
390			min	-.004	3	-.006	3	-.294	4	5.273e-6	12	NC	1	162.622	4
391		6	max	.004	1	.001	2	0	12	1.625e-3	4	NC	1	NC	1
392			min	-.004	3	-.005	3	-.261	4	4.758e-6	12	NC	1	183.637	4
393		7	max	.004	1	0	2	0	12	1.703e-3	4	NC	1	NC	1
394			min	-.004	3	-.005	3	-.228	4	4.243e-6	12	NC	1	209.896	4
395		8	max	.003	1	0	2	0	12	1.782e-3	4	NC	1	NC	1
396			min	-.003	3	-.005	3	-.197	4	3.727e-6	12	NC	1	243.316	4
397		9	max	.003	1	0	2	0	12	1.861e-3	4	NC	1	NC	1
398			min	-.003	3	-.004	3	-.167	4	3.212e-6	12	NC	1	286.781	4
399		10	max	.003	1	0	2	0	12	1.939e-3	4	NC	1	NC	1
400			min	-.003	3	-.004	3	-.139	4	2.696e-6	12	NC	1	344.815	4
401		11	max	.002	1	0	2	0	12	2.018e-3	4	NC	1	NC	1
402			min	-.003	3	-.004	3	-.113	4	2.181e-6	12	NC	1	424.84	4
403		12	max	.002	1	0	10	0	12	2.096e-3	4	NC	1	NC	1
404			min	-.002	3	-.003	3	-.089	4	1.665e-6	12	NC	1	539.74	4
405		13	max	.002	1	0	15	0	12	2.175e-3	4	NC	1	NC	1
406			min	-.002	3	-.003	3	-.067	4	1.15e-6	12	NC	1	713.539	4
407		14	max	.002	1	0	15	0	12	2.254e-3	4	NC	1	NC	1
408			min	-.002	3	-.003	3	-.048	4	4.508e-7	10	NC	1	995.401	4
409		15	max	.001	1	0	15	0	12	2.332e-3	4	NC	1	NC	1
410			min	-.001	3	-.002	3	-.032	4	-4.228e-6	1	NC	1	1499.583	4
411		16	max	0	1	0	15	0	12	2.411e-3	4	NC	1	NC	1
412			min	0	3	-.002	4	-.019	4	-1.567e-5	1	NC	1	2545.688	4
413		17	max	0	1	0	15	0	12	2.49e-3	4	NC	1	NC	1
414			min	0	3	-.001	4	-.009	4	-2.711e-5	1	NC	1	5341.774	4
415		18	max	0	1	0	15	0	12	2.568e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.003	4	-3.854e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.647e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-4.998e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.552e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-5.884e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	3.901e-7	4	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-3.988e-6	1	NC	1	NC	1
423		3	max	0	3	0	15	.025	4	5.892e-4	4	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.349e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.037	4	1.178e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-4.3e-5	1	NC	1	NC	1
427		5	max	0	3	-.002	15	.048	4	1.767e-3	4	NC	1	NC	1
428			min	0	2	-.007	4	-.001	1	-6.251e-5	1	NC	1	9519.987	4
429		6	max	.001	3	-.002	15	.059	4	2.355e-3	4	NC	1	NC	1
430			min	0	2	-.009	4	-.001	1	-8.202e-5	1	NC	1	8606.827	4
431		7	max	.001	3	-.003	15	.069	4	2.944e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.001	2	-.01	4	-.002	1	-1.015e-4	1	9107.966	4	8219.589	4
433		8	max	.002	3	-.003	15	.078	4	3.533e-3	4	NC	1	NC	1
434			min	-.001	2	-.012	4	-.002	1	-1.21e-4	1	8131.054	4	8206.774	4
435		9	max	.002	3	-.003	15	.088	4	4.122e-3	4	NC	1	NC	1
436			min	-.001	2	-.013	4	-.002	1	-1.405e-4	1	7548.418	4	8528.215	4
437		10	max	.002	3	-.003	15	.096	4	4.71e-3	4	NC	2	NC	1
438			min	-.002	2	-.013	4	-.003	1	-1.6e-4	1	7256.996	4	9216.805	4
439		11	max	.002	3	-.003	15	.105	4	5.299e-3	4	NC	2	NC	1
440			min	-.002	2	-.013	4	-.003	1	-1.796e-4	1	7213.281	4	NC	1
441		12	max	.003	3	-.003	15	.113	4	5.888e-3	4	NC	1	NC	1
442			min	-.002	2	-.013	4	-.003	1	-1.991e-4	1	7416.537	4	NC	1
443		13	max	.003	3	-.003	15	.121	4	6.477e-3	4	NC	1	NC	1
444			min	-.002	2	-.012	4	-.004	1	-2.186e-4	1	7910.597	4	NC	1
445		14	max	.003	3	-.003	15	.129	4	7.066e-3	4	NC	1	NC	1
446			min	-.002	2	-.011	4	-.004	1	-2.381e-4	1	8807.085	4	NC	1
447		15	max	.003	3	-.002	15	.137	4	7.654e-3	4	NC	1	NC	1
448			min	-.002	2	-.01	4	-.004	1	-2.576e-4	1	NC	1	NC	1
449		16	max	.003	3	-.002	15	.146	4	8.243e-3	4	NC	1	NC	1
450			min	-.003	2	-.008	4	-.005	1	-2.771e-4	1	NC	1	NC	1
451		17	max	.004	3	-.001	15	.154	4	8.832e-3	4	NC	1	NC	1
452			min	-.003	2	-.006	1	-.005	1	-2.966e-4	1	NC	1	NC	1
453		18	max	.004	3	0	15	.163	4	9.421e-3	4	NC	1	NC	1
454			min	-.003	2	-.005	1	-.005	1	-3.161e-4	1	NC	1	NC	1
455		19	max	.004	3	0	12	.173	4	1.001e-2	4	NC	1	NC	1
456			min	-.003	2	-.003	1	-.006	1	-3.356e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.002	2	.006	1	1.067e-5	1	NC	1	NC	2
458			min	0	3	-.004	3	-.173	4	-8.276e-4	4	NC	1	143.388	4
459		2	max	.003	1	.002	2	.005	1	1.067e-5	1	NC	1	NC	2
460			min	0	3	-.004	3	-.159	4	-8.276e-4	4	NC	1	156.105	4
461		3	max	.003	1	.002	2	.005	1	1.067e-5	1	NC	1	NC	2
462			min	0	3	-.004	3	-.145	4	-8.276e-4	4	NC	1	171.229	4
463		4	max	.003	1	.002	2	.004	1	1.067e-5	1	NC	1	NC	2
464			min	0	3	-.003	3	-.131	4	-8.276e-4	4	NC	1	189.387	4
465		5	max	.002	1	.002	2	.004	1	1.067e-5	1	NC	1	NC	2
466			min	0	3	-.003	3	-.117	4	-8.276e-4	4	NC	1	211.431	4
467		6	max	.002	1	.002	2	.004	1	1.067e-5	1	NC	1	NC	2
468			min	0	3	-.003	3	-.104	4	-8.276e-4	4	NC	1	238.542	4
469		7	max	.002	1	.002	2	.003	1	1.067e-5	1	NC	1	NC	2
470			min	0	3	-.003	3	-.091	4	-8.276e-4	4	NC	1	272.398	4
471		8	max	.002	1	.002	2	.003	1	1.067e-5	1	NC	1	NC	2
472			min	0	3	-.003	3	-.079	4	-8.276e-4	4	NC	1	315.445	4
473		9	max	.002	1	.001	2	.002	1	1.067e-5	1	NC	1	NC	1
474			min	0	3	-.002	3	-.067	4	-8.276e-4	4	NC	1	371.367	4
475		10	max	.002	1	.001	2	.002	1	1.067e-5	1	NC	1	NC	1
476			min	0	3	-.002	3	-.056	4	-8.276e-4	4	NC	1	445.922	4
477		11	max	.001	1	.001	2	.002	1	1.067e-5	1	NC	1	NC	1
478			min	0	3	-.002	3	-.045	4	-8.276e-4	4	NC	1	548.532	4
479		12	max	.001	1	0	2	.001	1	1.067e-5	1	NC	1	NC	1
480			min	0	3	-.002	3	-.036	4	-8.276e-4	4	NC	1	695.497	4
481		13	max	.001	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
482			min	0	3	-.001	3	-.027	4	-8.276e-4	4	NC	1	917.076	4
483		14	max	0	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
484			min	0	3	-.001	3	-.019	4	-8.276e-4	4	NC	1	1274.84	4
485		15	max	0	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
486			min	0	3	0	3	-.013	4	-8.276e-4	4	NC	1	1910.749	4
487		16	max	0	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
488			min	0	3	0	3	-.008	4	-8.276e-4	4	NC	1	3217.337	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
490		min	0	3	0	3	-.004	4	-8.276e-4	4	NC	1	6650.557	4
491	18	max	0	1	0	2	0	1	1.067e-5	1	NC	1	NC	1
492		min	0	3	0	3	-.001	4	-8.276e-4	4	NC	1	NC	1
493	19	max	0	1	0	1	0	1	1.067e-5	1	NC	1	NC	1
494		min	0	1	0	1	0	1	-8.276e-4	4	NC	1	NC	1
495	M1	1	max	.005	3	.122	.456	4	1.691e-2	1	NC	1	NC	1
496		min	-.002	2	-.025	3	0	12	-2.415e-2	3	NC	1	NC	1
497	2	max	.005	3	.06	1	.444	4	8.247e-3	1	NC	5	NC	1
498		min	-.002	2	-.012	3	-.004	1	-1.194e-2	3	1860.034	1	NC	1
499	3	max	.005	3	.008	3	.432	4	1.325e-2	4	NC	5	NC	1
500		min	-.002	2	-.007	2	-.006	1	-1.164e-4	3	890.959	1	9060.993	5
501	4	max	.005	3	.041	3	.42	4	1.169e-2	4	NC	5	NC	1
502		min	-.002	2	-.084	1	-.006	1	-4.526e-3	3	557.59	1	6152.177	5
503	5	max	.005	3	.083	3	.409	4	1.013e-2	1	NC	15	NC	1
504		min	-.002	2	-.165	1	-.004	1	-8.936e-3	3	399.504	1	4688.501	5
505	6	max	.005	3	.128	3	.397	4	1.525e-2	1	NC	15	NC	1
506		min	-.002	2	-.244	1	-.002	1	-1.335e-2	3	312.893	1	3830.498	5
507	7	max	.005	3	.172	3	.385	4	2.037e-2	1	9843.686	15	NC	1
508		min	-.002	2	-.315	1	0	12	-1.776e-2	3	261.999	1	3278.978	4
509	8	max	.005	3	.208	3	.372	4	2.549e-2	1	8748.632	15	NC	1
510		min	-.002	2	-.372	1	0	12	-2.217e-2	3	231.995	1	2908.35	4
511	9	max	.005	3	.232	3	.357	4	2.814e-2	1	8177.523	15	NC	1
512		min	-.002	2	-.408	1	0	1	-2.236e-2	3	216.412	1	2698.609	4
513	10	max	.004	3	.241	3	.34	4	2.916e-2	1	8003.558	15	NC	1
514		min	-.002	2	-.419	1	0	12	-1.975e-2	3	211.756	1	2641.128	4
515	11	max	.004	3	.235	3	.321	4	3.017e-2	1	8177.354	15	NC	1
516		min	-.002	2	-.407	1	0	12	-1.714e-2	3	216.728	1	2709.573	4
517	12	max	.004	3	.215	3	.301	4	2.854e-2	1	8748.243	15	NC	1
518		min	-.002	2	-.371	1	0	1	-1.442e-2	3	232.97	1	2921.258	5
519	13	max	.004	3	.183	3	.277	4	2.293e-2	1	9842.94	15	NC	1
520		min	-.002	2	-.313	1	0	1	-1.155e-2	3	264.393	1	3443.677	4
521	14	max	.004	3	.142	3	.252	4	1.731e-2	1	NC	15	NC	1
522		min	-.002	2	-.241	1	0	12	-8.671e-3	3	318.03	1	4504.287	4
523	15	max	.004	3	.096	3	.226	4	1.169e-2	1	NC	15	NC	1
524		min	-.002	2	-.161	1	0	12	-5.796e-3	3	410.095	1	6760.624	4
525	16	max	.004	3	.048	3	.2	4	8.838e-3	4	NC	5	NC	1
526		min	-.002	2	-.08	1	0	12	-2.92e-3	3	579.921	1	NC	1
527	17	max	.004	3	.003	3	.176	4	9.754e-3	4	NC	5	NC	1
528		min	-.002	2	-.005	2	0	12	-4.51e-5	3	941.529	1	NC	1
529	18	max	.004	3	.059	1	.156	4	1.042e-2	2	NC	5	NC	1
530		min	-.002	2	-.037	3	0	12	-4.277e-3	3	1988.584	1	NC	1
531	19	max	.004	3	.116	1	.138	4	2.096e-2	2	NC	1	NC	1
532		min	-.002	2	-.075	3	0	1	-8.675e-3	3	NC	1	NC	1
533	M5	1	max	.016	3	.284	.456	4	0	1	NC	1	NC	1
534		min	-.01	2	-.037	3	0	1	-2.311e-6	4	NC	1	NC	1
535	2	max	.016	3	.14	1	.447	4	6.783e-3	4	NC	5	NC	1
536		min	-.01	2	-.019	3	0	1	0	1	800.625	1	NC	1
537	3	max	.016	3	.024	3	.436	4	1.336e-2	4	NC	5	NC	1
538		min	-.01	2	-.023	1	0	1	0	1	374.301	1	7596.106	4
539	4	max	.016	3	.112	3	.424	4	1.088e-2	4	9627.536	15	NC	1
540		min	-.01	2	-.221	1	0	1	0	1	227.156	1	5499.987	4
541	5	max	.015	3	.23	3	.411	4	8.409e-3	4	6735.736	15	NC	1
542		min	-.01	2	-.438	1	0	1	0	1	158.802	1	4422.854	4
543	6	max	.015	3	.362	3	.398	4	5.935e-3	4	5185.056	15	NC	1
544		min	-.01	2	-.654	1	0	1	0	1	122.139	1	3758.684	4
545	7	max	.015	3	.49	3	.385	4	3.46e-3	4	4289.699	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.009	2	-.851	1	0	1	0	1	100.964	1	3296.495	4
547		8	max	.014	3	.597	3	.371	4	9.861e-4	4	3769.165	15	NC	1
548			min	-.009	2	-1.009	1	0	1	0	1	88.652	1	2942.825	4
549		9	max	.014	3	.667	3	.357	4	0	1	3502.247	15	NC	1
550			min	-.009	2	-1.108	1	0	1	-1.331e-6	5	82.342	1	2699.068	4
551		10	max	.014	3	.692	3	.34	4	0	1	3421.824	15	NC	1
552			min	-.009	2	-1.141	1	0	1	-1.259e-6	5	80.466	1	2660.335	4
553		11	max	.013	3	.674	3	.321	4	0	1	3502.309	15	NC	1
554			min	-.009	2	-1.108	1	0	1	-1.187e-6	5	82.469	1	2736.622	4
555		12	max	.013	3	.616	3	.301	4	7.025e-4	4	3769.313	15	NC	1
556			min	-.008	2	-1.006	1	0	1	0	1	89.072	1	2882.16	4
557		13	max	.013	3	.521	3	.278	4	2.465e-3	4	4290.002	15	NC	1
558			min	-.008	2	-.845	1	0	1	0	1	102.056	1	3389.94	4
559		14	max	.013	3	.402	3	.251	4	4.228e-3	4	5185.65	15	NC	1
560			min	-.008	2	-.643	1	0	1	0	1	124.598	1	4618.774	4
561		15	max	.012	3	.269	3	.224	4	5.991e-3	4	6736.912	15	NC	1
562			min	-.008	2	-.424	1	0	1	0	1	164.14	1	7789.254	5
563		16	max	.012	3	.134	3	.197	4	7.754e-3	4	9629.999	15	NC	1
564			min	-.008	2	-.207	1	0	1	0	1	239.125	1	NC	1
565		17	max	.012	3	.009	3	.173	4	9.517e-3	4	NC	5	NC	1
566			min	-.008	2	-.014	2	0	1	0	1	403.462	1	NC	1
567		18	max	.012	3	.138	1	.153	4	4.833e-3	4	NC	5	NC	1
568			min	-.008	2	-.098	3	0	1	0	1	878.869	1	NC	1
569		19	max	.012	3	.267	1	.138	4	0	1	NC	1	NC	1
570			min	-.008	2	-.193	3	0	1	-9.506e-7	4	NC	1	NC	1
571	M9	1	max	.005	3	.122	1	.456	4	2.415e-2	3	NC	1	NC	1
572			min	-.002	2	-.025	3	0	1	-1.691e-2	1	NC	1	NC	1
573		2	max	.005	3	.06	1	.446	4	1.194e-2	3	NC	5	NC	1
574			min	-.002	2	-.012	3	0	12	-8.247e-3	1	1860.034	1	NC	1
575		3	max	.005	3	.008	3	.435	4	1.332e-2	4	NC	5	NC	1
576			min	-.002	2	-.007	2	0	12	-3.332e-5	10	890.959	1	7760.005	4
577		4	max	.005	3	.041	3	.423	4	1.046e-2	5	NC	5	NC	1
578			min	-.002	2	-.084	1	0	12	-5.016e-3	1	557.59	1	5561.401	4
579		5	max	.005	3	.083	3	.411	4	8.936e-3	3	NC	15	NC	1
580			min	-.002	2	-.165	1	0	12	-1.013e-2	1	399.504	1	4433.132	4
581		6	max	.005	3	.128	3	.398	4	1.335e-2	3	NC	15	NC	1
582			min	-.002	2	-.244	1	0	12	-1.525e-2	1	312.893	1	3744.433	4
583		7	max	.005	3	.172	3	.385	4	1.776e-2	3	9831.184	15	NC	1
584			min	-.002	2	-.315	1	0	1	-2.037e-2	1	261.999	1	3275.488	4
585		8	max	.005	3	.208	3	.371	4	2.217e-2	3	8737.758	15	NC	1
586			min	-.002	2	-.372	1	0	1	-2.549e-2	1	231.995	1	2926.505	5
587		9	max	.005	3	.232	3	.357	4	2.236e-2	3	8167.478	15	NC	1
588			min	-.002	2	-.408	1	0	12	-2.814e-2	1	216.412	1	2692.712	4
589		10	max	.004	3	.241	3	.34	4	1.975e-2	3	7993.754	15	NC	1
590			min	-.002	2	-.419	1	0	1	-2.916e-2	1	211.756	1	2641.962	4
591		11	max	.004	3	.235	3	.321	4	1.714e-2	3	8167.308	15	NC	1
592			min	-.002	2	-.407	1	0	1	-3.017e-2	1	216.728	1	2716.84	4
593		12	max	.004	3	.215	3	.301	4	1.442e-2	3	8737.433	15	NC	1
594			min	-.002	2	-.371	1	0	12	-2.854e-2	1	232.97	1	2903.076	4
595		13	max	.004	3	.183	3	.277	4	1.155e-2	3	9830.664	15	NC	1
596			min	-.002	2	-.313	1	0	12	-2.293e-2	1	264.393	1	3443.77	4
597		14	max	.004	3	.142	3	.251	4	8.671e-3	3	NC	15	NC	1
598			min	-.002	2	-.241	1	-.001	1	-1.731e-2	1	318.03	1	4603.567	5
599		15	max	.004	3	.096	3	.224	4	5.796e-3	3	NC	15	NC	1
600			min	-.002	2	-.161	1	-.004	1	-1.169e-2	1	410.095	1	7268.025	5
601		16	max	.004	3	.048	3	.197	4	7.545e-3	5	NC	5	NC	1
602			min	-.002	2	-.08	1	-.005	1	-6.076e-3	1	579.921	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.004	3	.003	3	.173	4	9.547e-3	4	NC	5	NC	1
604		min	-.002	2	-.005	2	-.006	1	-4.593e-4	1	941.529	1	NC	1
605	18	max	.004	3	.059	1	.154	4	4.514e-3	5	NC	5	NC	1
606		min	-.002	2	-.037	3	-.004	1	-1.042e-2	2	1988.584	1	NC	1
607	19	max	.004	3	.116	1	.138	4	8.675e-3	3	NC	1	NC	1
608		min	-.002	2	-.075	3	0	12	-2.096e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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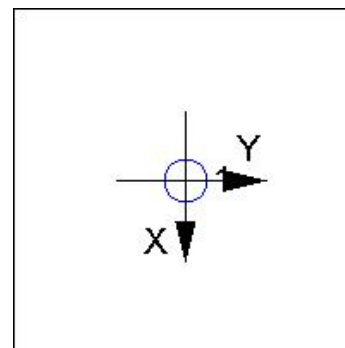
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

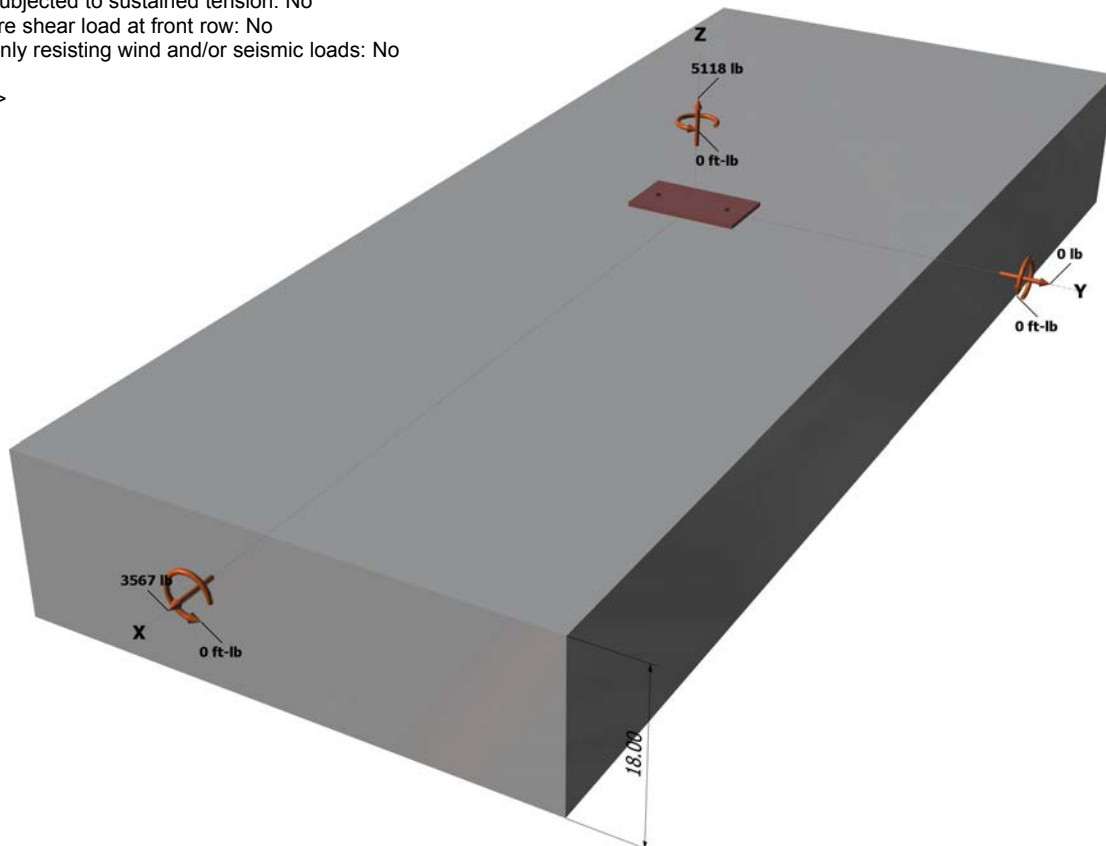
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

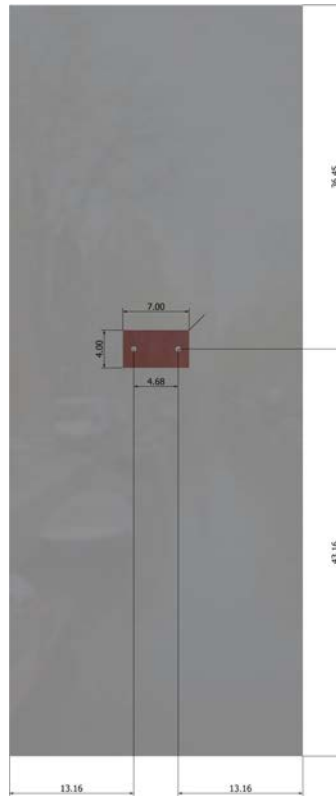
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Software  
Version 2.4.5673.0

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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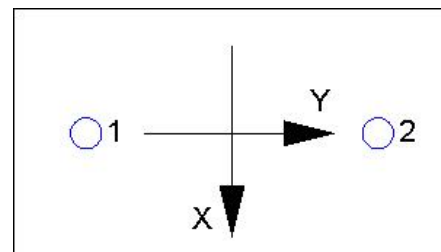
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5118  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
<b>Adhesive</b>	<b>5118</b>	<b>8093</b>	<b>0.63</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1784</b>	<b>3156</b>	<b>0.57</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.