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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.753 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>6%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.601 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>16%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.635 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<b>13%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.113 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>5%</b>



A cross brace kit is required every 37 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

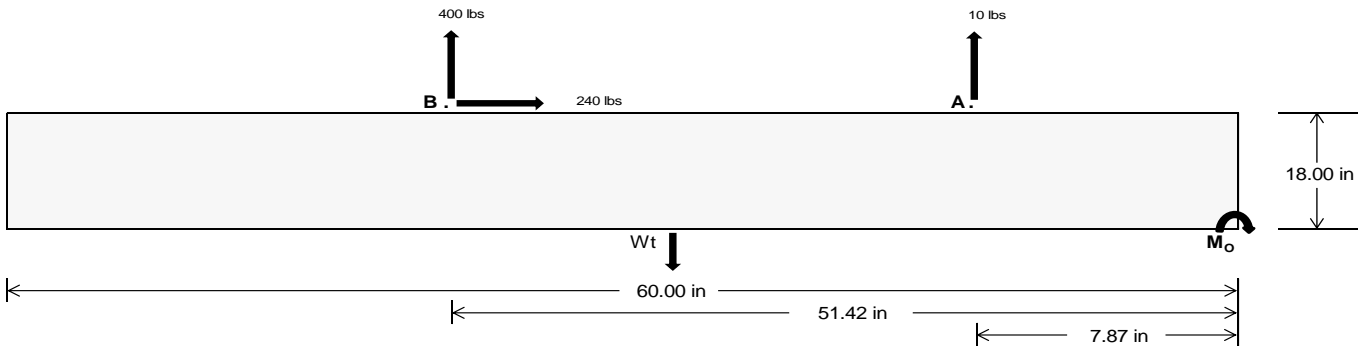
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>48.58</b>	<b>1735.74</b>	k
Compressive Load =	<b>978.93</b>	<b>1142.28</b>	k
Lateral Load =	<b>1.67</b>	<b>1041.87</b>	k
Moment (Weak Axis) =	<b>0.00</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 24964.1$  in-lbs  
Resisting Force Required = 832.14 lbs  
S.F. = 1.67  
Weight Required = 1386.90 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 240.35 lbs  
Friction = 0.4  
Weight Required = 600.88 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 240.35 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width	21 in	22 in	23 in	24 in
	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	322 lbs	322 lbs	322 lbs	322 lbs	373 lbs	373 lbs	373 lbs	373 lbs	492 lbs	492 lbs	492 lbs	492 lbs	-20 lbs	-20 lbs	-20 lbs	-20 lbs
$F_B$	217 lbs	217 lbs	217 lbs	217 lbs	486 lbs	486 lbs	486 lbs	486 lbs	507 lbs	507 lbs	507 lbs	507 lbs	-800 lbs	-800 lbs	-800 lbs	-800 lbs
$F_V$	29 lbs	29 lbs	29 lbs	29 lbs	430 lbs	430 lbs	430 lbs	430 lbs	342 lbs	342 lbs	342 lbs	342 lbs	-481 lbs	-481 lbs	-481 lbs	-481 lbs
$P_{total}$	2443 lbs	2533 lbs	2624 lbs	2714 lbs	2762 lbs	2853 lbs	2944 lbs	3034 lbs	2902 lbs	2992 lbs	3083 lbs	3173 lbs	322 lbs	377 lbs	431 lbs	485 lbs
$M$	250 lbs-ft	250 lbs-ft	250 lbs-ft	250 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	466 lbs-ft	516 lbs-ft	516 lbs-ft	516 lbs-ft	516 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft	669 lbs-ft
$e$	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	2.08 ft	1.78 ft	1.55 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	244.8 psf	243.6 psf	242.4 psf	241.4 psf	251.8 psf	250.3 psf	248.9 psf	247.5 psf	260.9 psf	258.9 psf	257.1 psf	255.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	313.5 psf	309.1 psf	305.2 psf	301.5 psf	379.6 psf	372.2 psf	365.5 psf	359.3 psf	402.3 psf	393.9 psf	386.3 psf	379.2 psf	290.5 psf	189.5 psf	158.3 psf	144.3 psf

Maximum Bearing Pressure = 402 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

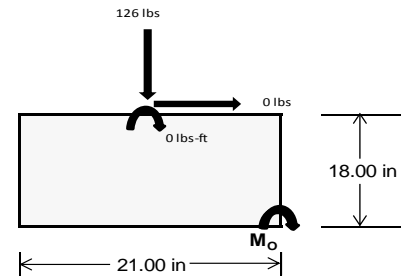
#### Overturning Check

$M_o = 0.0 \text{ ft-lbs}$   
 Resisting Force Required = 0.00 lbs  
 S.F. = 1.67  
 Weight Required = 0.00 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	52 lbs	126 lbs	50 lbs	162 lbs	448 lbs	159 lbs	15 lbs	37 lbs	14 lbs
$F_v$	0 lbs	0 lbs	0 lbs	1 lbs	0 lbs	0 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	2409 lbs	2482 lbs	2406 lbs	2405 lbs	2691 lbs	2402 lbs	704 lbs	726 lbs	703 lbs
$M$	0 lbs-ft	0 lbs-ft	0 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
$f_{min}$	275.2 sqft	283.6 sqft	274.9 sqft	274.5 sqft	307.4 sqft	274.3 sqft	80.5 sqft	82.9 sqft	80.4 sqft
$f_{max}$	275.3 psf	283.7 psf	275.0 psf	275.2 psf	307.7 psf	274.7 psf	80.5 psf	83.0 psf	80.4 psf



Maximum Bearing Pressure = 308 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.765 k
Allowable Uplift =	1.214 k
Utilization =	<u>63%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.095 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

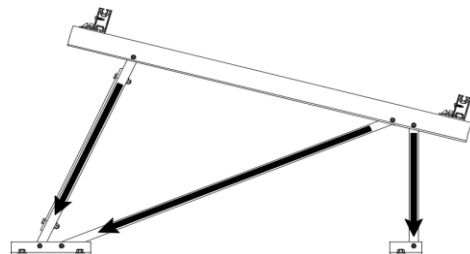
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.753 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>

#### Diagonal Strut

Maximum Axial Load =	0.601 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



#### Rear Strut

Maximum Axial Load =	1.098 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.113 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	32.32 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.646 in
Max Drift, $\Delta_{MAX}$ =	0.006 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$124.989$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 48.00 \text{ in}$$

$$J = 0.255$$

$$129.794$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.266 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &21.0529 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	228.145	2	280.456	2	.003	10	0	10	0	1	0	1
2		min	-268.874	3	-423.807	3	-.15	3	0	3	0	1	0	1
3	N7	max	.002	3	271.634	1	.016	10	0	10	0	1	0	1
4		min	-.13	2	3.287	12	-.539	1	0	1	0	1	0	1
5	N15	max	0	15	753.023	1	.148	9	0	9	0	1	0	1
6		min	-1.284	2	-37.369	3	-.688	3	-.001	3	0	1	0	1
7	N16	max	727.462	2	878.679	2	0	2	0	9	0	1	0	1
8		min	-801.436	3	-1335.184	3	-86.303	3	0	3	0	1	0	1
9	N23	max	.002	3	271.856	1	.746	1	.001	1	0	1	0	1
10		min	-.13	2	3.717	12	-.016	10	0	10	0	1	0	1
11	N24	max	228.145	2	282.87	2	87.045	3	0	9	0	1	0	1
12		min	-269.474	3	-423.063	3	-.003	10	0	3	0	1	0	1
13	Totals:	max	1182.208	2	2636.577	2	0	9						
14		min	-1339.836	3	-2211.349	3	0	3						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	202.226	2	.656	4	.111	1	0	10	0	15	0	1
2			min	-363.406	3	.154	15	-.073	3	0	1	0	1	0	1
3		2	max	202.352	2	.605	4	.111	1	0	10	0	15	0	15
4			min	-363.312	3	.142	15	-.073	3	0	1	0	3	0	4
5		3	max	202.477	2	.553	4	.111	1	0	10	0	15	0	15
6			min	-363.218	3	.13	15	-.073	3	0	1	0	3	0	4
7		4	max	202.603	2	.502	4	.111	1	0	10	0	9	0	15
8			min	-363.123	3	.118	15	-.073	3	0	1	0	3	0	4
9		5	max	202.729	2	.451	4	.111	1	0	10	0	9	0	15
10			min	-363.029	3	.106	15	-.073	3	0	1	0	3	0	4
11		6	max	202.855	2	.4	4	.111	1	0	10	0	9	0	15
12			min	-362.934	3	.094	15	-.073	3	0	1	0	3	0	4
13		7	max	202.981	2	.349	4	.111	1	0	10	0	9	0	15
14			min	-362.84	3	.082	15	-.073	3	0	1	0	3	0	4
15		8	max	203.107	2	.298	4	.111	1	0	10	0	9	0	15
16			min	-362.746	3	.07	15	-.073	3	0	1	0	3	0	4
17		9	max	203.233	2	.247	4	.111	1	0	10	0	1	0	15
18			min	-362.651	3	.058	15	-.073	3	0	1	0	3	0	4
19		10	max	203.358	2	.195	4	.111	1	0	10	0	1	0	15
20			min	-362.557	3	.046	15	-.073	3	0	1	0	3	0	4
21		11	max	203.484	2	.144	4	.111	1	0	10	0	1	0	15
22			min	-362.462	3	.033	12	-.073	3	0	1	0	3	0	4
23		12	max	203.61	2	.103	2	.111	1	0	10	0	1	0	15
24			min	-362.368	3	.013	12	-.073	3	0	1	0	3	0	4
25		13	max	203.736	2	.064	2	.111	1	0	10	0	1	0	15
26			min	-362.274	3	-.014	3	-.073	3	0	1	0	3	0	4
27		14	max	203.862	2	.024	2	.111	1	0	10	0	1	0	15
28			min	-362.179	3	-.044	3	-.073	3	0	1	0	3	0	4
29		15	max	203.988	2	-.014	15	.111	1	0	10	0	1	0	15
30			min	-362.085	3	-.074	3	-.073	3	0	1	0	3	0	4
31		16	max	204.114	2	-.026	15	.111	1	0	10	0	1	0	15
32			min	-361.99	3	-.111	4	-.073	3	0	1	0	3	0	4
33		17	max	204.24	2	-.038	15	.111	1	0	10	0	1	0	15
34			min	-361.896	3	-.163	4	-.073	3	0	1	0	3	0	4
35		18	max	204.365	2	-.05	15	.111	1	0	10	0	1	0	15
36			min	-361.802	3	-.214	4	-.073	3	0	1	0	3	0	4
37		19	max	204.491	2	-.062	15	.111	1	0	10	0	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-361.707	3	-.265	4	-.073	3	0	1	0	3	0	4
39	M3	1	max	187.255	2	1.759	4	.002	10	0	10	0	1	4
40		min	-177.951	3	.414	15	-.145	1	0	1	0	10	0	15
41		2	max	187.186	2	1.582	4	.002	10	0	10	0	1	2
42		min	-178.003	3	.372	15	-.145	1	0	1	0	10	0	12
43		3	max	187.116	2	1.405	4	.002	10	0	10	0	1	2
44		min	-178.055	3	.33	15	-.145	1	0	1	0	10	0	3
45		4	max	187.047	2	1.228	4	.002	10	0	10	0	1	15
46		min	-178.107	3	.289	15	-.145	1	0	1	0	10	0	4
47		5	max	186.978	2	1.051	4	.002	10	0	10	0	1	15
48		min	-178.159	3	.247	15	-.145	1	0	1	0	10	0	4
49		6	max	186.908	2	.875	4	.002	10	0	10	0	1	15
50		min	-178.211	3	.206	15	-.145	1	0	1	0	10	0	4
51		7	max	186.839	2	.698	4	.002	10	0	10	0	1	15
52		min	-178.263	3	.164	15	-.145	1	0	1	0	10	0	4
53		8	max	186.77	2	.521	4	.002	10	0	10	0	1	15
54		min	-178.315	3	.123	15	-.145	1	0	1	0	10	-.001	4
55		9	max	186.7	2	.344	4	.002	10	0	10	0	1	15
56		min	-178.367	3	.081	15	-.145	1	0	1	0	10	-.001	4
57		10	max	186.631	2	.167	4	.002	10	0	10	0	1	15
58		min	-178.419	3	.039	15	-.145	1	0	1	0	10	-.001	4
59		11	max	186.562	2	.019	2	.002	10	0	10	0	1	15
60		min	-178.471	3	-.038	3	-.145	1	0	1	0	10	-.001	4
61		12	max	186.492	2	-.044	15	.002	10	0	10	0	1	15
62		min	-178.523	3	-.186	4	-.145	1	0	1	0	10	-.001	4
63		13	max	186.423	2	-.085	15	.002	10	0	10	0	1	15
64		min	-178.575	3	-.363	4	-.145	1	0	1	0	10	-.001	4
65		14	max	186.354	2	-.127	15	.002	10	0	10	0	1	15
66		min	-178.627	3	-.54	4	-.145	1	0	1	0	10	-.001	4
67		15	max	186.284	2	-.168	15	.002	10	0	10	0	1	15
68		min	-178.679	3	-.717	4	-.145	1	0	1	0	10	0	4
69		16	max	186.215	2	-.21	15	.002	10	0	10	0	9	15
70		min	-178.731	3	-.894	4	-.145	1	0	1	0	11	0	4
71		17	max	186.146	2	-.252	15	.002	10	0	10	0	10	15
72		min	-178.783	3	-1.071	4	-.145	1	0	1	0	1	0	4
73		18	max	186.076	2	-.293	15	.002	10	0	10	0	10	15
74		min	-178.835	3	-1.247	4	-.145	1	0	1	0	1	0	4
75		19	max	186.007	2	-.335	15	.002	10	0	10	0	10	1
76		min	-178.887	3	-1.424	4	-.145	1	0	1	0	1	0	1
77	M4	1	max	270.469	1	0	1	.016	10	0	1	0	3	1
78		min	2.704	12	0	1	-.565	1	0	1	0	2	0	1
79		2	max	270.534	1	0	1	.016	10	0	1	0	15	1
80		min	2.737	12	0	1	-.565	1	0	1	0	1	0	1
81		3	max	270.599	1	0	1	.016	10	0	1	0	15	1
82		min	2.769	12	0	1	-.565	1	0	1	0	1	0	1
83		4	max	270.663	1	0	1	.016	10	0	1	0	10	1
84		min	2.801	12	0	1	-.565	1	0	1	0	1	0	1
85		5	max	270.728	1	0	1	.016	10	0	1	0	10	1
86		min	2.834	12	0	1	-.565	1	0	1	0	1	0	1
87		6	max	270.793	1	0	1	.016	10	0	1	0	10	1
88		min	2.866	12	0	1	-.565	1	0	1	0	1	0	1
89		7	max	270.857	1	0	1	.016	10	0	1	0	10	1
90		min	2.898	12	0	1	-.565	1	0	1	0	1	0	1
91		8	max	270.922	1	0	1	.016	10	0	1	0	10	1
92		min	2.931	12	0	1	-.565	1	0	1	0	1	0	1
93		9	max	270.987	1	0	1	.016	10	0	1	0	10	1
94		min	2.963	12	0	1	-.565	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	271.052	1	0	1	.016	10	0	1	0	10	0	1
96		min	2.996	12	0	1	-.565	1	0	1	0	1	0	1
97	11	max	271.116	1	0	1	.016	10	0	1	0	10	0	1
98		min	3.028	12	0	1	-.565	1	0	1	0	1	0	1
99	12	max	271.181	1	0	1	.016	10	0	1	0	10	0	1
100		min	3.06	12	0	1	-.565	1	0	1	0	1	0	1
101	13	max	271.246	1	0	1	.016	10	0	1	0	10	0	1
102		min	3.093	12	0	1	-.565	1	0	1	0	1	0	1
103	14	max	271.31	1	0	1	.016	10	0	1	0	10	0	1
104		min	3.125	12	0	1	-.565	1	0	1	0	1	0	1
105	15	max	271.375	1	0	1	.016	10	0	1	0	10	0	1
106		min	3.157	12	0	1	-.565	1	0	1	0	1	0	1
107	16	max	271.44	1	0	1	.016	10	0	1	0	10	0	1
108		min	3.19	12	0	1	-.565	1	0	1	0	1	0	1
109	17	max	271.504	1	0	1	.016	10	0	1	0	10	0	1
110		min	3.222	12	0	1	-.565	1	0	1	0	1	0	1
111	18	max	271.569	1	0	1	.016	10	0	1	0	10	0	1
112		min	3.254	12	0	1	-.565	1	0	1	0	1	0	1
113	19	max	271.634	1	0	1	.016	10	0	1	0	10	0	1
114		min	3.287	12	0	1	-.565	1	0	1	0	1	0	1
115	M6	1	max	632.538	2	.656	.023	9	0	3	0	3	0	1
116		min	-1097.981	3	.154	15	-.272	3	0	2	0	2	0	1
117	2	max	632.663	2	.605	4	.023	9	0	3	0	3	0	15
118		min	-1097.887	3	.142	15	-.272	3	0	2	0	2	0	4
119	3	max	632.789	2	.553	4	.023	9	0	3	0	3	0	15
120		min	-1097.792	3	.13	15	-.272	3	0	2	0	2	0	4
121	4	max	632.915	2	.502	4	.023	9	0	3	0	3	0	15
122		min	-1097.698	3	.118	15	-.272	3	0	2	0	2	0	4
123	5	max	633.041	2	.451	4	.023	9	0	3	0	3	0	15
124		min	-1097.603	3	.106	15	-.272	3	0	2	0	2	0	4
125	6	max	633.167	2	.407	2	.023	9	0	3	0	3	0	15
126		min	-1097.509	3	.086	12	-.272	3	0	2	0	2	0	4
127	7	max	633.293	2	.367	2	.023	9	0	3	0	9	0	15
128		min	-1097.415	3	.067	12	-.272	3	0	2	0	3	0	4
129	8	max	633.419	2	.327	2	.023	9	0	3	0	9	0	15
130		min	-1097.32	3	.047	12	-.272	3	0	2	0	3	0	4
131	9	max	633.545	2	.288	2	.023	9	0	3	0	9	0	15
132		min	-1097.226	3	.027	12	-.272	3	0	2	0	3	0	4
133	10	max	633.67	2	.248	2	.023	9	0	3	0	9	0	12
134		min	-1097.131	3	-.001	3	-.272	3	0	2	0	3	0	2
135	11	max	633.796	2	.208	2	.023	9	0	3	0	9	0	12
136		min	-1097.037	3	-.031	3	-.272	3	0	2	0	3	0	2
137	12	max	633.922	2	.168	2	.023	9	0	3	0	9	0	12
138		min	-1096.943	3	-.061	3	-.272	3	0	2	0	3	0	2
139	13	max	634.048	2	.128	2	.023	9	0	3	0	9	0	12
140		min	-1096.848	3	-.091	3	-.272	3	0	2	0	3	0	2
141	14	max	634.174	2	.088	2	.023	9	0	3	0	9	0	12
142		min	-1096.754	3	-.121	3	-.272	3	0	2	0	3	0	2
143	15	max	634.3	2	.048	2	.023	9	0	3	0	9	0	12
144		min	-1096.659	3	-.151	3	-.272	3	0	2	0	3	0	2
145	16	max	634.426	2	.009	2	.023	9	0	3	0	9	0	12
146		min	-1096.565	3	-.181	3	-.272	3	0	2	0	3	0	2
147	17	max	634.551	2	-.031	2	.023	9	0	3	0	9	0	12
148		min	-1096.471	3	-.211	3	-.272	3	0	2	0	3	0	2
149	18	max	634.677	2	-.05	15	.023	9	0	3	0	9	0	3
150		min	-1096.376	3	-.24	3	-.272	3	0	2	0	3	0	2
151	19	max	634.803	2	-.062	15	.023	9	0	3	0	9	0	3





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152			min	-1096.282	3	- .27	3	- .272	3	0	2	0	3	0	2
153	M7	1	max	601.396	2	1.761	4	.037	3	0	9	0	1	0	2
154			min	-501.204	3	.414	15	-.014	1	0	3	0	3	0	3
155		2	max	601.327	2	1.584	4	.037	3	0	9	0	1	0	2
156			min	-501.256	3	.372	15	-.014	1	0	3	0	3	0	3
157		3	max	601.257	2	1.407	4	.037	3	0	9	0	1	0	2
158			min	-501.308	3	.331	15	-.014	1	0	3	0	3	0	3
159		4	max	601.188	2	1.23	4	.037	3	0	9	0	1	0	2
160			min	-501.36	3	.289	15	-.014	1	0	3	0	3	0	3
161		5	max	601.119	2	1.054	4	.037	3	0	9	0	1	0	15
162			min	-501.412	3	.248	15	-.014	1	0	3	0	3	0	3
163		6	max	601.049	2	.877	4	.037	3	0	9	0	1	0	15
164			min	-501.464	3	.206	15	-.014	1	0	3	0	3	0	4
165		7	max	600.98	2	.7	4	.037	3	0	9	0	1	0	15
166			min	-501.516	3	.165	15	-.014	1	0	3	0	3	0	4
167		8	max	600.911	2	.523	4	.037	3	0	9	0	1	0	15
168			min	-501.568	3	.123	15	-.014	1	0	3	0	3	-.001	4
169		9	max	600.841	2	.35	2	.037	3	0	9	0	1	0	15
170			min	-501.62	3	.078	12	-.014	1	0	3	0	3	-.001	4
171		10	max	600.772	2	.212	2	.037	3	0	9	0	1	0	15
172			min	-501.672	3	.004	3	-.014	1	0	3	0	3	-.001	4
173		11	max	600.703	2	.074	2	.037	3	0	9	0	1	0	15
174			min	-501.724	3	-.1	3	-.014	1	0	3	0	3	-.001	4
175		12	max	600.633	2	-.043	15	.037	3	0	9	0	1	0	15
176			min	-501.776	3	-.203	3	-.014	1	0	3	0	3	-.001	4
177		13	max	600.564	2	-.085	15	.037	3	0	9	0	1	0	15
178			min	-501.828	3	-.361	4	-.014	1	0	3	0	3	-.001	4
179		14	max	600.495	2	-.126	15	.037	3	0	9	0	1	0	15
180			min	-501.88	3	-.538	4	-.014	1	0	3	0	3	-.001	4
181		15	max	600.425	2	-.168	15	.037	3	0	9	0	1	0	15
182			min	-501.932	3	-.715	4	-.014	1	0	3	0	3	0	4
183		16	max	600.356	2	-.21	15	.037	3	0	9	0	1	0	15
184			min	-501.984	3	-.892	4	-.014	1	0	3	0	3	0	4
185		17	max	600.287	2	-.251	15	.037	3	0	9	0	1	0	15
186			min	-502.036	3	-1.068	4	-.014	1	0	3	0	3	0	4
187		18	max	600.217	2	-.293	15	.037	3	0	9	0	9	0	15
188			min	-502.088	3	-1.245	4	-.014	1	0	3	0	3	0	4
189		19	max	600.148	2	-.334	15	.037	3	0	9	0	9	0	1
190			min	-502.14	3	-1.422	4	-.014	1	0	3	0	3	0	1
191	M8	1	max	751.859	1	0	1	.156	9	0	1	0	2	0	1
192			min	-38.243	3	0	1	-.683	3	0	1	0	3	0	1
193		2	max	751.923	1	0	1	.156	9	0	1	0	9	0	1
194			min	-38.194	3	0	1	-.683	3	0	1	0	3	0	1
195		3	max	751.988	1	0	1	.156	9	0	1	0	9	0	1
196			min	-38.146	3	0	1	-.683	3	0	1	0	3	0	1
197		4	max	752.053	1	0	1	.156	9	0	1	0	9	0	1
198			min	-38.097	3	0	1	-.683	3	0	1	0	3	0	1
199		5	max	752.117	1	0	1	.156	9	0	1	0	9	0	1
200			min	-38.049	3	0	1	-.683	3	0	1	0	3	0	1
201		6	max	752.182	1	0	1	.156	9	0	1	0	9	0	1
202			min	-38	3	0	1	-.683	3	0	1	0	3	0	1
203		7	max	752.247	1	0	1	.156	9	0	1	0	9	0	1
204			min	-37.952	3	0	1	-.683	3	0	1	0	3	0	1
205		8	max	752.312	1	0	1	.156	9	0	1	0	9	0	1
206			min	-37.903	3	0	1	-.683	3	0	1	0	3	0	1
207		9	max	752.376	1	0	1	.156	9	0	1	0	9	0	1
208			min	-37.855	3	0	1	-.683	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	752.441	1	0	1	.156	9	0	1	0	9	0	1
210		min	-37.806	3	0	1	-.683	3	0	1	0	3	0	1
211	11	max	752.506	1	0	1	.156	9	0	1	0	9	0	1
212		min	-37.758	3	0	1	-.683	3	0	1	0	3	0	1
213	12	max	752.57	1	0	1	.156	9	0	1	0	9	0	1
214		min	-37.709	3	0	1	-.683	3	0	1	0	3	0	1
215	13	max	752.635	1	0	1	.156	9	0	1	0	9	0	1
216		min	-37.661	3	0	1	-.683	3	0	1	0	3	0	1
217	14	max	752.7	1	0	1	.156	9	0	1	0	9	0	1
218		min	-37.612	3	0	1	-.683	3	0	1	0	3	0	1
219	15	max	752.765	1	0	1	.156	9	0	1	0	9	0	1
220		min	-37.563	3	0	1	-.683	3	0	1	0	3	0	1
221	16	max	752.829	1	0	1	.156	9	0	1	0	9	0	1
222		min	-37.515	3	0	1	-.683	3	0	1	0	3	0	1
223	17	max	752.894	1	0	1	.156	9	0	1	0	9	0	1
224		min	-37.466	3	0	1	-.683	3	0	1	0	3	0	1
225	18	max	752.959	1	0	1	.156	9	0	1	0	9	0	1
226		min	-37.418	3	0	1	-.683	3	0	1	-.001	3	0	1
227	19	max	753.023	1	0	1	.156	9	0	1	0	9	0	1
228		min	-37.369	3	0	1	-.683	3	0	1	-.001	3	0	1
229	M10	1	max	203.493	2	.656	4	0	10	0	1	0	1	1
230		min	-291.773	3	.154	15	-.106	1	0	3	0	3	0	1
231	2	max	203.619	2	.605	4	0	10	0	1	0	1	0	15
232		min	-291.678	3	.142	15	-.106	1	0	3	0	3	0	4
233	3	max	203.745	2	.553	4	0	10	0	1	0	1	0	15
234		min	-291.584	3	.13	15	-.106	1	0	3	0	3	0	4
235	4	max	203.87	2	.502	4	0	10	0	1	0	1	0	15
236		min	-291.49	3	.118	15	-.106	1	0	3	0	3	0	4
237	5	max	203.996	2	.451	4	0	10	0	1	0	1	0	15
238		min	-291.395	3	.106	15	-.106	1	0	3	0	3	0	4
239	6	max	204.122	2	.4	4	0	10	0	1	0	1	0	15
240		min	-291.301	3	.094	15	-.106	1	0	3	0	3	0	4
241	7	max	204.248	2	.349	4	0	10	0	1	0	1	0	15
242		min	-291.206	3	.082	15	-.106	1	0	3	0	3	0	4
243	8	max	204.374	2	.298	4	0	10	0	1	0	9	0	15
244		min	-291.112	3	.07	15	-.106	1	0	3	0	3	0	4
245	9	max	204.5	2	.247	4	0	10	0	1	0	10	0	15
246		min	-291.018	3	.058	15	-.106	1	0	3	0	3	0	4
247	10	max	204.626	2	.195	4	0	10	0	1	0	10	0	15
248		min	-290.923	3	.046	15	-.106	1	0	3	0	3	0	4
249	11	max	204.752	2	.144	4	0	10	0	1	0	10	0	15
250		min	-290.829	3	.034	15	-.106	1	0	3	0	3	0	4
251	12	max	204.877	2	.103	2	0	10	0	1	0	10	0	15
252		min	-290.734	3	.022	15	-.106	1	0	3	0	3	0	4
253	13	max	205.003	2	.064	2	0	10	0	1	0	10	0	15
254		min	-290.64	3	.001	3	-.106	1	0	3	0	3	0	4
255	14	max	205.129	2	.024	2	0	10	0	1	0	10	0	15
256		min	-290.546	3	-.029	3	-.106	1	0	3	0	3	0	4
257	15	max	205.255	2	-.014	15	0	10	0	1	0	10	0	15
258		min	-290.451	3	-.06	4	-.106	1	0	3	0	3	0	4
259	16	max	205.381	2	-.026	15	0	10	0	1	0	10	0	15
260		min	-290.357	3	-.111	4	-.106	1	0	3	0	3	0	4
261	17	max	205.507	2	-.038	15	0	10	0	1	0	10	0	15
262		min	-290.262	3	-.163	4	-.106	1	0	3	0	3	0	4
263	18	max	205.633	2	-.05	15	0	10	0	1	0	10	0	15
264		min	-290.168	3	-.214	4	-.106	1	0	3	0	3	0	4
265	19	max	205.759	2	-.062	15	0	10	0	1	0	10	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-290.074	3	-.265	4	-.106	1	0	3	0	3	0	4
267	M11	1	max	186.827	2	1.759	4	.151	1	0	3	0	3	0	4
268			min	-178.778	3	.414	15	-.055	3	0	10	0	1	0	15
269		2	max	186.758	2	1.582	4	.151	1	0	3	0	3	0	2
270			min	-178.83	3	.372	15	-.055	3	0	10	0	1	0	3
271		3	max	186.689	2	1.405	4	.151	1	0	3	0	3	0	2
272			min	-178.882	3	.33	15	-.055	3	0	10	0	1	0	3
273		4	max	186.619	2	1.228	4	.151	1	0	3	0	3	0	15
274			min	-178.934	3	.289	15	-.055	3	0	10	0	1	0	4
275		5	max	186.55	2	1.051	4	.151	1	0	3	0	3	0	15
276			min	-178.986	3	.247	15	-.055	3	0	10	0	1	0	4
277		6	max	186.481	2	.875	4	.151	1	0	3	0	3	0	15
278			min	-179.038	3	.206	15	-.055	3	0	10	0	1	0	4
279		7	max	186.411	2	.698	4	.151	1	0	3	0	3	0	15
280			min	-179.09	3	.164	15	-.055	3	0	10	0	1	0	4
281		8	max	186.342	2	.521	4	.151	1	0	3	0	3	0	15
282			min	-179.142	3	.123	15	-.055	3	0	10	0	1	-.001	4
283		9	max	186.273	2	.344	4	.151	1	0	3	0	3	0	15
284			min	-179.194	3	.081	15	-.055	3	0	10	0	1	-.001	4
285		10	max	186.203	2	.167	4	.151	1	0	3	0	3	0	15
286			min	-179.246	3	.039	15	-.055	3	0	10	0	1	-.001	4
287		11	max	186.134	2	.019	2	.151	1	0	3	0	3	0	15
288			min	-179.298	3	-.045	3	-.055	3	0	10	0	1	-.001	4
289		12	max	186.065	2	-.044	15	.151	1	0	3	0	3	0	15
290			min	-179.35	3	-.186	4	-.055	3	0	10	0	1	-.001	4
291		13	max	185.995	2	-.085	15	.151	1	0	3	0	3	0	15
292			min	-179.402	3	-.363	4	-.055	3	0	10	0	1	-.001	4
293		14	max	185.926	2	-.127	15	.151	1	0	3	0	3	0	15
294			min	-179.454	3	-.54	4	-.055	3	0	10	0	1	-.001	4
295		15	max	185.857	2	-.168	15	.151	1	0	3	0	3	0	15
296			min	-179.506	3	-.717	4	-.055	3	0	10	0	1	0	4
297		16	max	185.788	2	-.21	15	.151	1	0	3	0	3	0	15
298			min	-179.558	3	-.894	4	-.055	3	0	10	0	10	0	4
299		17	max	185.718	2	-.252	15	.151	1	0	3	0	3	0	15
300			min	-179.61	3	-1.071	4	-.055	3	0	10	0	10	0	4
301		18	max	185.649	2	-.293	15	.151	1	0	3	0	3	0	15
302			min	-179.662	3	-1.247	4	-.055	3	0	10	0	10	0	4
303		19	max	185.58	2	-.335	15	.151	1	0	3	0	3	0	1
304			min	-179.714	3	-1.424	4	-.055	3	0	10	0	10	0	1
305	M12	1	max	270.691	1	0	1	.781	1	0	1	0	2	0	1
306			min	3.134	12	0	1	-.016	10	0	1	0	3	0	1
307		2	max	270.756	1	0	1	.781	1	0	1	0	1	0	1
308			min	3.167	12	0	1	-.016	10	0	1	0	15	0	1
309		3	max	270.82	1	0	1	.781	1	0	1	0	1	0	1
310			min	3.199	12	0	1	-.016	10	0	1	0	10	0	1
311		4	max	270.885	1	0	1	.781	1	0	1	0	1	0	1
312			min	3.231	12	0	1	-.016	10	0	1	0	10	0	1
313		5	max	270.95	1	0	1	.781	1	0	1	0	1	0	1
314			min	3.264	12	0	1	-.016	10	0	1	0	10	0	1
315		6	max	271.014	1	0	1	.781	1	0	1	0	1	0	1
316			min	3.296	12	0	1	-.016	10	0	1	0	10	0	1
317		7	max	271.079	1	0	1	.781	1	0	1	0	1	0	1
318			min	3.328	12	0	1	-.016	10	0	1	0	10	0	1
319		8	max	271.144	1	0	1	.781	1	0	1	0	1	0	1
320			min	3.361	12	0	1	-.016	10	0	1	0	10	0	1
321		9	max	271.209	1	0	1	.781	1	0	1	0	1	0	1
322			min	3.393	12	0	1	-.016	10	0	1	0	10	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323		10	max	271.273	1	0	1	.781	1	0	1	0	1	0	1
324			min	3.426	12	0	1	-.016	10	0	1	0	10	0	1
325		11	max	271.338	1	0	1	.781	1	0	1	0	1	0	1
326			min	3.458	12	0	1	-.016	10	0	1	0	10	0	1
327		12	max	271.403	1	0	1	.781	1	0	1	0	1	0	1
328			min	3.49	12	0	1	-.016	10	0	1	0	10	0	1
329		13	max	271.467	1	0	1	.781	1	0	1	0	1	0	1
330			min	3.523	12	0	1	-.016	10	0	1	0	10	0	1
331		14	max	271.532	1	0	1	.781	1	0	1	0	1	0	1
332			min	3.555	12	0	1	-.016	10	0	1	0	10	0	1
333		15	max	271.597	1	0	1	.781	1	0	1	0	1	0	1
334			min	3.587	12	0	1	-.016	10	0	1	0	10	0	1
335		16	max	271.661	1	0	1	.781	1	0	1	.001	1	0	1
336			min	3.62	12	0	1	-.016	10	0	1	0	10	0	1
337		17	max	271.726	1	0	1	.781	1	0	1	.001	1	0	1
338			min	3.652	12	0	1	-.016	10	0	1	0	10	0	1
339		18	max	271.791	1	0	1	.781	1	0	1	.001	1	0	1
340			min	3.684	12	0	1	-.016	10	0	1	0	10	0	1
341		19	max	271.856	1	0	1	.781	1	0	1	.001	1	0	1
342			min	3.717	12	0	1	-.016	10	0	1	0	10	0	1
343	M1	1	max	76.151	1	342.466	3	.243	10	0	2	.036	1	0	2
344			min	3.159	15	-222.182	2	-18.198	1	0	3	0	10	0	3
345		2	max	76.291	1	342.285	3	.243	10	0	2	.032	1	.048	2
346			min	3.201	15	-222.424	2	-18.198	1	0	3	0	10	-.075	3
347		3	max	90.115	3	4.254	9	.244	10	0	10	.028	1	.096	2
348			min	-16.15	10	-24.984	2	-18.139	1	0	1	0	10	-.147	3
349		4	max	90.22	3	4.053	9	.244	10	0	10	.024	1	.101	2
350			min	-16.034	10	-25.226	2	-18.139	1	0	1	0	10	-.145	3
351		5	max	90.325	3	3.851	9	.244	10	0	10	.02	1	.107	2
352			min	-15.917	10	-25.468	2	-18.139	1	0	1	0	10	-.142	3
353		6	max	90.43	3	3.65	9	.244	10	0	10	.016	1	.112	2
354			min	-15.801	10	-25.709	2	-18.139	1	0	1	0	10	-.139	3
355		7	max	90.534	3	3.448	9	.244	10	0	10	.012	1	.118	2
356			min	-15.685	10	-25.951	2	-18.139	1	0	1	0	10	-.136	3
357		8	max	90.639	3	3.247	9	.244	10	0	10	.008	1	.124	2
358			min	-15.568	10	-26.193	2	-18.139	1	0	1	0	10	-.133	3
359		9	max	90.744	3	3.045	9	.244	10	0	10	.004	1	.129	2
360			min	-15.452	10	-26.435	2	-18.139	1	0	1	0	10	-.13	3
361		10	max	90.848	3	2.844	9	.244	10	0	10	.002	3	.135	2
362			min	-15.336	10	-26.677	2	-18.139	1	0	1	0	10	-.127	3
363		11	max	90.953	3	2.642	9	.244	10	0	10	0	3	.141	2
364			min	-15.219	10	-26.919	2	-18.139	1	0	1	-.004	1	-.124	3
365		12	max	91.058	3	2.441	9	.244	10	0	10	0	10	.147	2
366			min	-15.103	10	-27.16	2	-18.139	1	0	1	-.008	1	-.121	3
367		13	max	91.163	3	2.239	9	.244	10	0	10	0	10	.153	2
368			min	-14.986	10	-27.402	2	-18.139	1	0	1	-.012	1	-.118	3
369		14	max	91.267	3	2.038	9	.244	10	0	10	0	10	.159	2
370			min	-14.87	10	-27.644	2	-18.139	1	0	1	-.016	1	-.115	3
371		15	max	91.372	3	1.836	9	.244	10	0	10	0	10	.165	2
372			min	-14.754	10	-27.886	2	-18.139	1	0	1	-.02	1	-.111	3
373		16	max	89.21	2	127.08	2	.245	10	0	1	0	10	.169	2
374			min	-6.025	3	-165.094	3	-18.267	1	0	3	-.024	1	-.107	3
375		17	max	89.35	2	126.838	2	.245	10	0	1	0	10	.142	2
376			min	-5.92	3	-165.275	3	-18.267	1	0	3	-.028	1	-.071	3
377		18	max	-3.2	15	328.284	2	.261	10	0	3	0	10	.072	2
378			min	-76.288	1	-163.003	3	-18.85	1	0	2	-.032	1	-.036	3
379		19	max	-3.157	15	328.042	2	.261	10	0	3	0	10	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-76.148	1	-163.185	3	-18.85	1	0	2	-.036	1	0	3
381	M5	max	187.508	1	1093.472	3	0	11	0	9	.011	3	0	3
382		min	-2.055	3	-700.884	2	-78.214	3	0	3	0	11	0	2
383		max	187.647	1	1093.291	3	0	11	0	9	0	9	.151	2
384		min	-1.95	3	-701.126	2	-78.214	3	0	3	-.006	3	-.237	3
385		max	250.018	3	4.843	9	8.466	3	0	3	0	9	.301	2
386		min	-52.123	2	-83.958	2	-.181	9	0	9	-.022	3	-.469	3
387		max	250.123	3	4.641	9	8.466	3	0	3	0	9	.319	2
388		min	-51.983	2	-84.2	2	-.181	9	0	9	-.02	3	-.458	3
389		max	250.227	3	4.44	9	8.466	3	0	3	0	9	.338	2
390		min	-51.844	2	-84.442	2	-.181	9	0	9	-.018	3	-.447	3
391		max	250.332	3	4.238	9	8.466	3	0	3	0	9	.356	2
392		min	-51.704	2	-84.684	2	-.181	9	0	9	-.016	3	-.436	3
393		max	250.437	3	4.037	9	8.466	3	0	3	0	9	.374	2
394		min	-51.564	2	-84.926	2	-.181	9	0	9	-.014	3	-.425	3
395		max	250.541	3	3.835	9	8.466	3	0	3	0	9	.393	2
396		min	-51.425	2	-85.167	2	-.181	9	0	9	-.013	3	-.414	3
397		max	250.646	3	3.634	9	8.466	3	0	3	0	9	.411	2
398		min	-51.285	2	-85.409	2	-.181	9	0	9	-.011	3	-.403	3
399		max	250.751	3	3.432	9	8.466	3	0	3	0	2	.43	2
400		min	-51.145	2	-85.651	2	-.181	9	0	9	-.009	3	-.392	3
401		max	250.856	3	3.231	9	8.466	3	0	3	0	2	.448	2
402		min	-51.006	2	-85.893	2	-.181	9	0	9	-.007	3	-.381	3
403		max	250.96	3	3.029	9	8.466	3	0	3	0	2	.467	2
404		min	-50.866	2	-86.135	2	-.181	9	0	9	-.005	3	-.37	3
405		max	251.065	3	2.828	9	8.466	3	0	3	0	2	.486	2
406		min	-50.727	2	-86.377	2	-.181	9	0	9	-.003	3	-.359	3
407		max	251.17	3	2.626	9	8.466	3	0	3	0	2	.505	2
408		min	-50.587	2	-86.618	2	-.181	9	0	9	-.002	3	-.347	3
409		max	251.274	3	2.425	9	8.466	3	0	3	0	3	.523	2
410		min	-50.447	2	-86.86	2	-.181	9	0	9	0	9	-.336	3
411		max	279.681	2	417.219	2	8.438	3	0	3	.002	3	.538	2
412		min	-23.72	3	-472.187	3	-.181	9	0	2	0	9	-.321	3
413		max	279.821	2	416.978	2	8.438	3	0	3	.003	3	.447	2
414		min	-23.615	3	-472.368	3	-.181	9	0	2	0	9	-.219	3
415		max	-3.381	12	1038.759	2	7.757	3	0	3	.005	3	.225	2
416		min	-187.659	1	-505.855	3	-.033	9	0	9	0	9	-.109	3
417		max	-3.311	12	1038.518	2	7.757	3	0	3	.007	3	0	3
418		min	-187.52	1	-506.036	3	-.033	9	0	9	0	9	0	2
419	M9	max	76.064	1	342.375	3	83.161	3	0	3	0	10	0	2
420		min	3.15	15	-222.182	2	-.243	10	0	2	-.036	1	0	3
421		max	76.203	1	342.194	3	83.161	3	0	3	0	10	.048	2
422		min	3.192	15	-222.424	2	-.243	10	0	2	-.032	1	-.075	3
423		max	89.579	3	4.243	9	17.958	1	0	1	.015	3	.096	2
424		min	-15.795	10	-24.957	2	-2.712	3	0	10	-.027	1	-.147	3
425		max	89.684	3	4.042	9	17.958	1	0	1	.015	3	.101	2
426		min	-15.679	10	-25.199	2	-2.712	3	0	10	-.023	1	-.144	3
427		max	89.788	3	3.84	9	17.958	1	0	1	.014	3	.107	2
428		min	-15.563	10	-25.441	2	-2.712	3	0	10	-.02	1	-.142	3
429		max	89.893	3	3.639	9	17.958	1	0	1	.014	3	.112	2
430		min	-15.446	10	-25.683	2	-2.712	3	0	10	-.016	1	-.139	3
431		max	89.998	3	3.437	9	17.958	1	0	1	.013	3	.118	2
432		min	-15.33	10	-25.924	2	-2.712	3	0	10	-.012	1	-.136	3
433		max	90.103	3	3.236	9	17.958	1	0	1	.012	3	.124	2
434		min	-15.214	10	-26.166	2	-2.712	3	0	10	-.008	1	-.133	3
435		max	90.207	3	3.034	9	17.958	1	0	1	.012	3	.129	2
436		min	-15.097	10	-26.408	2	-2.712	3	0	10	-.004	1	-.13	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	90.312	3	2.833	9	17.958	1	0	1	.011	3	.135	2
438		min	-14.981	10	-26.65	2	-2.712	3	0	10	0	1	-.127	3
439	11	max	90.417	3	2.631	9	17.958	1	0	1	.011	3	.141	2
440		min	-14.865	10	-26.892	2	-2.712	3	0	10	0	10	-.124	3
441	12	max	90.521	3	2.43	9	17.958	1	0	1	.01	3	.147	2
442		min	-14.748	10	-27.133	2	-2.712	3	0	10	0	10	-.121	3
443	13	max	90.626	3	2.228	9	17.958	1	0	1	.012	1	.153	2
444		min	-14.632	10	-27.375	2	-2.712	3	0	10	0	10	-.118	3
445	14	max	90.731	3	2.026	9	17.958	1	0	1	.016	1	.159	2
446		min	-14.515	10	-27.617	2	-2.712	3	0	10	0	10	-.115	3
447	15	max	90.836	3	1.825	9	17.958	1	0	1	.019	1	.165	2
448		min	-14.399	10	-27.859	2	-2.712	3	0	10	0	10	-.111	3
449	16	max	89.401	2	126.75	2	18.09	1	0	10	.024	1	.169	2
450		min	-7.01	3	-165.671	3	-2.762	3	0	3	0	10	-.107	3
451	17	max	89.541	2	126.508	2	18.09	1	0	10	.027	1	.142	2
452		min	-6.906	3	-165.852	3	-2.762	3	0	3	0	10	-.071	3
453	18	max	-3.191	15	328.284	2	18.881	1	0	2	.032	1	.072	2
454		min	-76.194	1	-162.992	3	-2.29	3	0	3	0	10	-.036	3
455	19	max	-3.149	15	328.042	2	18.881	1	0	2	.036	1	0	2
456		min	-76.055	1	-163.173	3	-2.29	3	0	3	0	10	0	3
457	M13	1	max	83.154	3	222.097	2	-3.15	15	0	.036	1	0	2
458		min	-.243	10	-342.428	3	-76.059	1	0	3	0	10	0	3
459	2	max	83.154	3	158.53	2	-2.39	15	0	2	.015	3	.13	3
460		min	-.243	10	-243.636	3	-57.186	1	0	3	-.003	10	-.085	2
461	3	max	83.154	3	94.962	2	-1.63	15	0	2	.011	3	.217	3
462		min	-.243	10	-144.844	3	-38.312	1	0	3	-.015	1	-.141	2
463	4	max	83.154	3	31.394	2	.269	10	0	2	.008	3	.259	3
464		min	-.243	10	-46.052	3	-19.439	1	0	3	-.028	1	-.169	2
465	5	max	83.154	3	52.74	3	3.331	2	0	2	.005	3	.257	3
466		min	-.243	10	-32.173	2	-6.141	3	0	3	-.032	1	-.169	2
467	6	max	83.154	3	151.531	3	18.308	1	0	2	.002	3	.212	3
468		min	-.243	10	-95.741	2	-5.036	3	0	3	-.029	1	-.14	2
469	7	max	83.154	3	250.323	3	37.182	1	0	2	0	3	.123	3
470		min	-.243	10	-159.309	2	-3.93	3	0	3	-.016	1	-.084	2
471	8	max	83.154	3	349.115	3	56.055	1	0	2	.006	2	.003	1
472		min	-.243	10	-222.876	2	-2.825	3	0	3	-.001	3	-.01	3
473	9	max	83.154	3	447.907	3	74.929	1	0	2	.034	1	.114	2
474		min	-.243	10	-286.444	2	-1.719	3	0	3	-.002	3	-.188	3
475	10	max	83.154	3	-7.275	15	93.802	1	0	2	.071	1	.256	2
476		min	-.243	10	-546.699	3	.528	12	0	3	-.014	3	-.409	3
477	11	max	18.231	1	286.444	2	2.578	3	0	3	.033	1	.114	2
478		min	-.243	10	-447.907	3	-74.842	1	0	2	-.013	3	-.188	3
479	12	max	18.231	1	222.876	2	3.683	3	0	3	.006	2	.003	1
480		min	-.243	10	-349.115	3	-55.968	1	0	2	-.012	3	-.01	3
481	13	max	18.231	1	159.309	2	4.789	3	0	3	0	10	.123	3
482		min	-.243	10	-250.323	3	-37.095	1	0	2	-.016	1	-.084	2
483	14	max	18.231	1	95.741	2	5.895	3	0	3	-.001	15	.212	3
484		min	-.243	10	-151.531	3	-18.221	1	0	2	-.029	1	-.14	2
485	15	max	18.231	1	32.173	2	7	3	0	3	-.001	15	.257	3
486		min	-.243	10	-52.739	3	-3.331	2	0	2	-.032	1	-.169	2
487	16	max	18.231	1	46.052	3	19.526	1	0	3	0	12	.259	3
488		min	-.243	10	-31.394	2	-.268	10	0	2	-.028	1	-.169	2
489	17	max	18.231	1	144.844	3	38.399	1	0	3	.003	3	.217	3
490		min	-.243	10	-94.962	2	1.639	15	0	2	-.015	1	-.141	2
491	18	max	18.231	1	243.636	3	57.273	1	0	3	.007	3	.13	3
492		min	-.243	10	-158.53	2	2.399	15	0	2	-.003	10	-.085	2
493	19	max	18.231	1	342.428	3	76.146	1	0	3	.036	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	-2.243	10	-222.097	2	3.159	15	0	2	0	10	0	3
495	M16	1	max	2.294	3	328.149	2	-3.149	15	0	3	.036	1	0	2
496			min	-18.847	1	-163.202	3	-76.06	1	0	2	0	10	0	3
497		2	max	2.294	3	233.932	2	-2.389	15	0	3	.006	1	.062	3
498			min	-18.847	1	-116.953	3	-57.186	1	0	2	-.003	10	-.125	2
499		3	max	2.294	3	139.714	2	-1.63	15	0	3	0	3	.104	3
500			min	-18.847	1	-70.704	3	-38.313	1	0	2	-.015	1	-.208	2
501		4	max	2.294	3	45.497	2	.255	10	0	3	-.001	15	.125	3
502			min	-18.847	1	-24.455	3	-19.439	1	0	2	-.028	1	-.249	2
503		5	max	2.294	3	21.795	3	3.307	2	0	3	-.001	15	.126	3
504			min	-18.847	1	-48.72	2	-4.024	3	0	2	-.032	1	-.248	2
505		6	max	2.294	3	68.044	3	18.308	1	0	3	-.001	15	.106	3
506			min	-18.847	1	-142.938	2	-2.919	3	0	2	-.029	1	-.206	2
507		7	max	2.294	3	114.293	3	37.181	1	0	3	0	10	.065	3
508			min	-18.847	1	-237.155	2	-1.813	3	0	2	-.016	1	-.121	2
509		8	max	2.294	3	160.542	3	56.055	1	0	3	.006	2	.005	2
510			min	-18.847	1	-331.373	2	-.708	3	0	2	-.008	3	0	15
511		9	max	2.294	3	206.791	3	74.928	1	0	3	.034	1	.173	2
512			min	-18.847	1	-425.59	2	.398	3	0	2	-.008	3	-.077	3
513		10	max	.261	10	-7.272	15	93.802	1	0	15	.071	1	.383	2
514			min	-18.847	1	-519.808	2	-2.83	3	0	2	-.007	3	-.18	3
515		11	max	.261	10	425.59	2	-1.314	12	0	2	.033	1	.173	2
516			min	-18.816	1	-206.791	3	-74.835	1	0	3	-.001	3	-.077	3
517		12	max	.261	10	331.373	2	-.577	12	0	2	.006	2	.005	2
518			min	-18.816	1	-160.542	3	-55.961	1	0	3	-.002	3	0	15
519		13	max	.261	10	237.155	2	.487	3	0	2	0	10	.065	3
520			min	-18.816	1	-114.293	3	-37.088	1	0	3	-.016	1	-.121	2
521		14	max	.261	10	142.938	2	1.592	3	0	2	-.001	12	.106	3
522			min	-18.816	1	-68.044	3	-18.214	1	0	3	-.029	1	-.206	2
523		15	max	.261	10	48.72	2	2.698	3	0	2	0	12	.126	3
524			min	-18.816	1	-21.795	3	-3.307	2	0	3	-.032	1	-.248	2
525		16	max	.261	10	24.455	3	19.533	1	0	2	0	3	.125	3
526			min	-18.816	1	-45.497	2	-.255	10	0	3	-.028	1	-.249	2
527		17	max	.261	10	70.704	3	38.406	1	0	2	.003	3	.104	3
528			min	-18.816	1	-139.714	2	1.638	15	0	3	-.015	1	-.208	2
529		18	max	.261	10	116.953	3	57.28	1	0	2	.006	1	.062	3
530			min	-18.816	1	-233.932	2	2.398	15	0	3	-.003	10	-.125	2
531		19	max	.261	10	163.202	3	76.153	1	0	2	.036	1	0	2
532			min	-18.816	1	-328.149	2	3.157	15	0	3	0	10	0	3
533	M15	1	max	0	1	.835	3	.136	3	0	1	0	1	0	1
534			min	-112.072	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.742	3	.136	3	0	1	0	1	0	1
536			min	-112.143	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.649	3	.136	3	0	1	0	1	0	1
538			min	-112.213	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.556	3	.136	3	0	1	0	1	0	1
540			min	-112.284	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.464	3	.136	3	0	1	0	1	0	1
542			min	-112.354	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.371	3	.136	3	0	1	0	1	0	1
544			min	-112.425	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.278	3	.136	3	0	1	0	3	0	1
546			min	-112.495	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.185	3	.136	3	0	1	0	3	0	1
548			min	-112.566	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.093	3	.136	3	0	1	0	3	0	1
550			min	-112.636	3	0	1	0	1	0	3	0	1	-.001	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	1	0	1	.136	3	0	1	0	3	0	1
552		min	-112.707	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.136	3	0	1	0	3	0	1
554		min	-112.777	3	-.093	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.136	3	0	1	0	3	0	1
556		min	-112.848	3	-.185	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.136	3	0	1	0	3	0	1
558		min	-112.918	3	-.278	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.136	3	0	1	0	3	0	1
560		min	-112.989	3	-.371	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.136	3	0	1	0	3	0	1
562		min	-113.059	3	-.464	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.136	3	0	1	0	3	0	1
564		min	-113.13	3	-.556	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.136	3	0	1	0	3	0	1
566		min	-113.2	3	-.649	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.136	3	0	1	0	3	0	1
568		min	-113.271	3	-.742	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.136	3	0	1	0	3	0	1
570		min	-113.341	3	-.835	3	0	1	0	3	0	1	0	1
571	M16A 1	max	0	2	1.428	4	.038	1	0	3	0	3	0	1
572		min	-111.736	3	0	2	-.056	3	0	1	0	1	0	1
573	2	max	0	2	1.269	4	.038	1	0	3	0	3	0	2
574		min	-111.665	3	0	2	-.056	3	0	1	0	1	0	4
575	3	max	0	2	1.111	4	.038	1	0	3	0	3	0	2
576		min	-111.595	3	0	2	-.056	3	0	1	0	1	0	4
577	4	max	0	2	.952	4	.038	1	0	3	0	3	0	2
578		min	-111.524	3	0	2	-.056	3	0	1	0	1	-.001	4
579	5	max	0	2	.793	4	.038	1	0	3	0	3	0	2
580		min	-111.454	3	0	2	-.056	3	0	1	0	1	-.001	4
581	6	max	0	2	.635	4	.038	1	0	3	0	3	0	2
582		min	-111.383	3	0	2	-.056	3	0	1	0	1	-.001	4
583	7	max	0	2	.476	4	.038	1	0	3	0	3	0	2
584		min	-111.313	3	0	2	-.056	3	0	1	0	1	-.002	4
585	8	max	0	2	.317	4	.038	1	0	3	0	3	0	2
586		min	-111.242	3	0	2	-.056	3	0	1	0	1	-.002	4
587	9	max	0	2	.159	4	.038	1	0	3	0	3	0	2
588		min	-111.172	3	0	2	-.056	3	0	1	0	1	-.002	4
589	10	max	0	2	0	1	.038	1	0	3	0	3	0	2
590		min	-111.101	3	0	1	-.056	3	0	1	0	1	-.002	4
591	11	max	0	2	0	2	.038	1	0	3	0	3	0	2
592		min	-111.031	3	-.159	4	-.056	3	0	1	0	1	-.002	4
593	12	max	.067	13	0	2	.038	1	0	3	0	3	0	2
594		min	-110.96	3	-.317	4	-.056	3	0	1	0	1	-.002	4
595	13	max	.164	13	0	2	.038	1	0	3	0	1	0	2
596		min	-110.89	3	-.476	4	-.056	3	0	1	0	4	-.002	4
597	14	max	.261	13	0	2	.038	1	0	3	0	1	0	2
598		min	-110.819	3	-.635	4	-.056	3	0	1	0	3	-.001	4
599	15	max	.358	13	0	2	.038	1	0	3	0	1	0	2
600		min	-110.749	3	-.793	4	-.056	3	0	1	0	3	-.001	4
601	16	max	.455	13	0	2	.038	1	0	3	0	1	0	2
602		min	-110.678	3	-.952	4	-.056	3	0	1	0	3	-.001	4
603	17	max	.569	4	0	2	.038	1	0	3	0	1	0	2
604		min	-110.608	3	-1.111	4	-.056	3	0	1	0	3	0	4
605	18	max	.69	4	0	2	.038	1	0	3	0	1	0	2
606		min	-110.537	3	-1.269	4	-.056	3	0	1	0	3	0	4
607	19	max	.81	4	0	2	.038	1	0	3	0	1	0	1





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-110.467	3	-1.428	4	-.056	3	0	1	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.009	2	.003	1	4.504e-6	10	NC	3	NC	1	
2			min	-.004	3	-.009	3	-.002	3	-2.913e-4	1	4188.08	2	NC	1	
3			2	max	.002	2	.009	2	.003	1	4.29e-6	10	NC	3	NC	1
4				min	-.003	3	-.009	3	-.002	3	-2.78e-4	1	4570.535	2	NC	1
5			3	max	.002	2	.008	2	.002	1	4.076e-6	10	NC	3	NC	1
6				min	-.003	3	-.009	3	-.002	3	-2.648e-4	1	5025.344	2	NC	1
7			4	max	.002	2	.007	2	.002	1	3.861e-6	10	NC	1	NC	1
8				min	-.003	3	-.008	3	-.002	3	-2.515e-4	1	5569.952	2	NC	1
9			5	max	.002	2	.006	2	.002	1	3.647e-6	10	NC	1	NC	1
10				min	-.003	3	-.008	3	-.001	3	-2.382e-4	1	6227.603	2	NC	1
11		6	max	.001	2	.006	2	.002	1	3.433e-6	10	NC	1	NC	1	
12			min	-.003	3	-.008	3	-.001	3	-2.25e-4	1	7029.782	2	NC	1	
13		7	max	.001	2	.005	2	.002	1	3.219e-6	10	NC	1	NC	1	
14			min	-.002	3	-.007	3	-.001	3	-2.117e-4	1	8019.939	2	NC	1	
15		8	max	.001	2	.004	2	.001	1	3.004e-6	10	NC	1	NC	1	
16			min	-.002	3	-.007	3	0	3	-1.984e-4	1	9259.388	2	NC	1	
17		9	max	.001	2	.004	2	.001	1	2.79e-6	10	NC	1	NC	1	
18			min	-.002	3	-.006	3	0	3	-1.852e-4	1	NC	1	NC	1	
19		10	max	0	2	.003	2	.001	1	2.576e-6	10	NC	1	NC	1	
20			min	-.002	3	-.006	3	0	3	-1.719e-4	1	NC	1	NC	1	
21		11	max	0	2	.003	2	0	1	2.362e-6	10	NC	1	NC	1	
22			min	-.002	3	-.005	3	0	3	-1.586e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	0	1	2.147e-6	10	NC	1	NC	1	
24			min	-.001	3	-.005	3	0	3	-1.454e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	1	1.933e-6	10	NC	1	NC	1	
26			min	-.001	3	-.004	3	0	3	-1.321e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	1	1.719e-6	10	NC	1	NC	1	
28			min	0	3	-.003	3	0	3	-1.188e-4	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	1	1.504e-6	10	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-1.056e-4	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	1	1.29e-6	10	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-9.231e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	1	1.076e-6	10	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-7.904e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	1	8.617e-7	10	NC	1	NC	1	
36			min	0	3	0	3	0	3	-6.578e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	6.474e-7	10	NC	1	NC	1	
38			min	0	1	0	1	0	1	-5.251e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	2.485e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-3.079e-7	10	NC	1	NC	1	
41			2	max	0	3	0	2	0	10	3.308e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-3.968e-7	10	NC	1	NC	1
43			3	max	0	3	0	2	0	10	4.131e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	-4.857e-7	10	NC	1	NC	1
45			4	max	0	3	0	2	0	3	4.954e-5	1	NC	1	NC	1
46				min	0	2	-.003	3	0	9	-5.746e-7	10	NC	1	NC	1
47			5	max	0	3	0	2	0	3	5.777e-5	1	NC	1	NC	1
48				min	0	2	-.003	3	0	9	-6.635e-7	10	NC	1	NC	1
49			6	max	0	3	0	2	0	3	6.6e-5	1	NC	1	NC	1
50				min	0	2	-.004	3	0	9	-7.524e-7	10	NC	1	NC	1
51		7	max	0	3	0	2	0	3	7.423e-5	1	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	9	-8.412e-7	10	NC	1	NC	1
53		8	max	0	3	0	2	0	3	8.246e-5	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-9.301e-7	10	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	9.07e-5	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-1.019e-6	10	NC	1	NC	1
57		10	max	.001	3	.002	2	0	1	9.893e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-1.108e-6	10	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	1.072e-4	1	NC	1	NC	1
60			min	-.001	2	-.007	3	0	10	-1.197e-6	10	NC	1	NC	1
61		12	max	.001	3	.003	2	0	1	1.154e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	10	-1.286e-6	10	NC	1	NC	1
63		13	max	.001	3	.003	2	0	1	1.236e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	10	-1.375e-6	10	NC	1	NC	1
65		14	max	.001	3	.004	2	.001	1	1.319e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	10	-1.464e-6	10	NC	1	NC	1
67		15	max	.002	3	.005	2	.001	1	1.401e-4	1	NC	1	NC	1
68			min	-.002	2	-.008	3	0	10	-1.552e-6	10	9076.236	2	NC	1
69		16	max	.002	3	.006	2	.002	1	1.483e-4	1	NC	1	NC	1
70			min	-.002	2	-.008	3	0	10	-1.641e-6	10	7682.131	2	NC	1
71		17	max	.002	3	.007	2	.002	1	1.565e-4	1	NC	1	NC	1
72			min	-.002	2	-.008	3	0	10	-1.73e-6	10	6605.083	2	NC	1
73		18	max	.002	3	.008	2	.002	1	1.648e-4	1	NC	1	NC	1
74			min	-.002	2	-.008	3	0	10	-1.819e-6	10	5763.382	2	NC	1
75		19	max	.002	3	.009	2	.002	1	1.73e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-1.908e-6	10	5099.653	2	NC	1
77	M4	1	max	.001	1	.011	2	0	10	2.697e-6	10	NC	1	NC	1
78			min	0	12	-.009	3	-.002	1	-2.195e-4	1	NC	1	NC	1
79		2	max	.001	1	.01	2	0	10	2.697e-6	10	NC	1	NC	1
80			min	0	12	-.009	3	-.002	1	-2.195e-4	1	NC	1	NC	1
81		3	max	.001	1	.01	2	0	10	2.697e-6	10	NC	1	NC	1
82			min	0	12	-.008	3	-.002	1	-2.195e-4	1	NC	1	NC	1
83		4	max	.001	1	.009	2	0	10	2.697e-6	10	NC	1	NC	1
84			min	0	12	-.008	3	-.001	1	-2.195e-4	1	NC	1	NC	1
85		5	max	.001	1	.008	2	0	10	2.697e-6	10	NC	1	NC	1
86			min	0	12	-.007	3	-.001	1	-2.195e-4	1	NC	1	NC	1
87		6	max	0	1	.008	2	0	10	2.697e-6	10	NC	1	NC	1
88			min	0	12	-.007	3	-.001	1	-2.195e-4	1	NC	1	NC	1
89		7	max	0	1	.007	2	0	10	2.697e-6	10	NC	1	NC	1
90			min	0	12	-.006	3	0	1	-2.195e-4	1	NC	1	NC	1
91		8	max	0	1	.007	2	0	10	2.697e-6	10	NC	1	NC	1
92			min	0	12	-.006	3	0	1	-2.195e-4	1	NC	1	NC	1
93		9	max	0	1	.006	2	0	10	2.697e-6	10	NC	1	NC	1
94			min	0	12	-.005	3	0	1	-2.195e-4	1	NC	1	NC	1
95		10	max	0	1	.005	2	0	10	2.697e-6	10	NC	1	NC	1
96			min	0	12	-.005	3	0	1	-2.195e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	10	2.697e-6	10	NC	1	NC	1
98			min	0	12	-.004	3	0	1	-2.195e-4	1	NC	1	NC	1
99		12	max	0	1	.004	2	0	10	2.697e-6	10	NC	1	NC	1
100			min	0	12	-.004	3	0	1	-2.195e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	10	2.697e-6	10	NC	1	NC	1
102			min	0	12	-.003	3	0	1	-2.195e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	10	2.697e-6	10	NC	1	NC	1
104			min	0	12	-.003	3	0	1	-2.195e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	10	2.697e-6	10	NC	1	NC	1
106			min	0	12	-.002	3	0	1	-2.195e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	10	2.697e-6	10	NC	1	NC	1
108			min	0	12	-.002	3	0	1	-2.195e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	10	2.697e-6	10	NC	1	NC	1
110			min	0	12	-.001	3	0	1	-2.195e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	2.697e-6	10	NC	1	NC	1
112			min	0	12	0	3	0	1	-2.195e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	2.697e-6	10	NC	1	NC	1
114			min	0	1	0	1	0	1	-2.195e-4	1	NC	1	NC	1
115	M6	1	max	.006	2	.03	2	0	9	4.83e-4	3	NC	3	NC	1
116			min	-.011	3	-.028	3	-.006	3	-8.418e-8	2	1310.583	2	6193.628	3
117		2	max	.006	2	.028	2	0	9	4.681e-4	3	NC	3	NC	1
118			min	-.01	3	-.027	3	-.006	3	-9.632e-7	1	1403.083	2	6571.734	3
119		3	max	.005	2	.026	2	0	9	4.533e-4	3	NC	3	NC	1
120			min	-.009	3	-.025	3	-.006	3	-2.419e-6	1	1509.175	2	7021.708	3
121		4	max	.005	2	.024	2	0	9	4.385e-4	3	NC	3	NC	1
122			min	-.009	3	-.024	3	-.005	3	-3.875e-6	1	1631.585	2	7558.815	3
123		5	max	.005	2	.022	2	0	9	4.237e-4	3	NC	3	NC	1
124			min	-.008	3	-.022	3	-.005	3	-5.33e-6	1	1773.83	2	8203.336	3
125		6	max	.004	2	.02	2	0	9	4.089e-4	3	NC	3	NC	1
126			min	-.008	3	-.021	3	-.004	3	-6.786e-6	1	1940.512	2	8982.593	3
127		7	max	.004	2	.018	2	0	9	3.94e-4	3	NC	3	NC	1
128			min	-.007	3	-.019	3	-.004	3	-8.242e-6	1	2137.776	2	9934.042	3
129		8	max	.004	2	.017	2	0	9	3.792e-4	3	NC	3	NC	1
130			min	-.007	3	-.018	3	-.004	3	-9.697e-6	1	2374.003	2	NC	1
131		9	max	.003	2	.015	2	0	9	3.644e-4	3	NC	3	NC	1
132			min	-.006	3	-.016	3	-.003	3	-1.115e-5	1	2660.931	2	NC	1
133		10	max	.003	2	.013	2	0	9	3.496e-4	3	NC	3	NC	1
134			min	-.005	3	-.015	3	-.003	3	-1.261e-5	1	3015.515	2	NC	1
135		11	max	.003	2	.011	2	0	9	3.348e-4	3	NC	3	NC	1
136			min	-.005	3	-.013	3	-.002	3	-1.406e-5	1	3463.181	2	NC	1
137		12	max	.002	2	.01	2	0	9	3.199e-4	3	NC	3	NC	1
138			min	-.004	3	-.011	3	-.002	3	-1.552e-5	1	4043.88	2	NC	1
139		13	max	.002	2	.008	2	0	9	3.051e-4	3	NC	3	NC	1
140			min	-.004	3	-.01	3	-.002	3	-1.698e-5	1	4824.172	2	NC	1
141		14	max	.002	2	.007	2	0	9	2.903e-4	3	NC	1	NC	1
142			min	-.003	3	-.008	3	-.001	3	-1.843e-5	1	5923.842	2	NC	1
143		15	max	.001	2	.005	2	0	9	2.755e-4	3	NC	1	NC	1
144			min	-.002	3	-.007	3	0	3	-1.989e-5	1	7582.402	2	NC	1
145		16	max	.001	2	.004	2	0	9	2.607e-4	3	NC	1	NC	1
146			min	-.002	3	-.005	3	0	3	-2.134e-5	1	NC	1	NC	1
147		17	max	0	2	.002	2	0	1	2.458e-4	3	NC	1	NC	1
148			min	-.001	3	-.003	3	0	3	-2.28e-5	1	NC	1	NC	1
149		18	max	0	2	.001	2	0	1	2.31e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-2.425e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	2.162e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-2.571e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	1.21e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-1.015e-4	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.12e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-7.631e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.029e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-5.116e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	.001	3	9.377e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-2.601e-5	3	NC	1	NC	1
161		5	max	.001	3	.005	2	.002	3	8.468e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	-8.603e-7	3	8968.057	2	NC	1
163		6	max	.002	3	.006	2	.002	3	2.429e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	1	0	2	7184.156	2	NC	1
165		7	max	.002	3	.008	2	.002	3	4.944e-5	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166		min	-.002	2	-.011	3	0	1	0	5	5963.445	2	NC	1
167	8	max	.002	3	.009	2	.003	3	7.459e-5	3	NC	3	NC	1
168		min	-.003	2	-.013	3	0	1	-2.442e-7	4	5068.095	2	NC	1
169	9	max	.003	3	.011	2	.003	3	9.973e-5	3	NC	3	NC	1
170		min	-.003	2	-.014	3	0	1	-1.257e-6	9	4379.616	2	NC	1
171	10	max	.003	3	.012	2	.003	3	1.249e-4	3	NC	3	NC	1
172		min	-.003	2	-.016	3	0	1	-2.501e-6	9	3832.322	2	NC	1
173	11	max	.003	3	.014	2	.003	3	1.5e-4	3	NC	3	NC	1
174		min	-.004	2	-.017	3	0	9	-3.745e-6	9	3386.746	2	NC	1
175	12	max	.003	3	.015	2	.003	3	1.752e-4	3	NC	3	NC	1
176		min	-.004	2	-.018	3	0	9	-4.989e-6	9	3017.606	2	NC	1
177	13	max	.004	3	.017	2	.003	3	2.003e-4	3	NC	3	NC	1
178		min	-.005	2	-.019	3	0	9	-6.233e-6	9	2707.821	2	NC	1
179	14	max	.004	3	.019	2	.003	3	2.255e-4	3	NC	3	NC	1
180		min	-.005	2	-.021	3	0	9	-7.477e-6	9	2445.332	2	NC	1
181	15	max	.004	3	.021	2	.003	3	2.506e-4	3	NC	3	NC	1
182		min	-.005	2	-.022	3	0	9	-8.721e-6	9	2221.299	2	NC	1
183	16	max	.005	3	.023	2	.003	3	2.758e-4	3	NC	3	NC	1
184		min	-.006	2	-.023	3	0	9	-9.965e-6	9	2029.046	2	NC	1
185	17	max	.005	3	.025	2	.003	3	3.009e-4	3	NC	3	NC	1
186		min	-.006	2	-.024	3	0	9	-1.121e-5	9	1863.406	2	NC	1
187	18	max	.005	3	.027	2	.003	3	3.261e-4	3	NC	3	NC	1
188		min	-.006	2	-.024	3	0	9	-1.245e-5	9	1720.302	2	NC	1
189	19	max	.006	3	.029	2	.003	3	3.512e-4	3	NC	3	NC	1
190		min	-.007	2	-.025	3	0	9	-1.37e-5	9	1596.473	2	NC	1
191	M8	1	max	.004	1	.034	2	0	-1.09e-7	10	NC	1	NC	1
192		min	0	3	-.028	3	-.002	3	-2.595e-4	3	NC	1	8947.938	3
193	2	max	.003	1	.032	2	0	9	-1.09e-7	10	NC	1	NC	1
194		min	0	3	-.027	3	-.002	3	-2.595e-4	3	NC	1	9756.113	3
195	3	max	.003	1	.03	2	0	9	-1.09e-7	10	NC	1	NC	1
196		min	0	3	-.025	3	-.002	3	-2.595e-4	3	NC	1	NC	1
197	4	max	.003	1	.029	2	0	9	-1.09e-7	10	NC	1	NC	1
198		min	0	3	-.023	3	-.002	3	-2.595e-4	3	NC	1	NC	1
199	5	max	.003	1	.027	2	0	9	-1.09e-7	10	NC	1	NC	1
200		min	0	3	-.022	3	-.001	3	-2.595e-4	3	NC	1	NC	1
201	6	max	.003	1	.025	2	0	9	-1.09e-7	10	NC	1	NC	1
202		min	0	3	-.02	3	-.001	3	-2.595e-4	3	NC	1	NC	1
203	7	max	.002	1	.023	2	0	9	-1.09e-7	10	NC	1	NC	1
204		min	0	3	-.019	3	-.001	3	-2.595e-4	3	NC	1	NC	1
205	8	max	.002	1	.021	2	0	9	-1.09e-7	10	NC	1	NC	1
206		min	0	3	-.017	3	0	3	-2.595e-4	3	NC	1	NC	1
207	9	max	.002	1	.019	2	0	9	-1.09e-7	10	NC	1	NC	1
208		min	0	3	-.016	3	0	3	-2.595e-4	3	NC	1	NC	1
209	10	max	.002	1	.017	2	0	9	-1.09e-7	10	NC	1	NC	1
210		min	0	3	-.014	3	0	3	-2.595e-4	3	NC	1	NC	1
211	11	max	.002	1	.015	2	0	9	-1.09e-7	10	NC	1	NC	1
212		min	0	3	-.013	3	0	3	-2.595e-4	3	NC	1	NC	1
213	12	max	.001	1	.013	2	0	9	-1.09e-7	10	NC	1	NC	1
214		min	0	3	-.011	3	0	3	-2.595e-4	3	NC	1	NC	1
215	13	max	.001	1	.011	2	0	9	-1.09e-7	10	NC	1	NC	1
216		min	0	3	-.009	3	0	3	-2.595e-4	3	NC	1	NC	1
217	14	max	0	1	.01	2	0	9	-1.09e-7	10	NC	1	NC	1
218		min	0	3	-.008	3	0	3	-2.595e-4	3	NC	1	NC	1
219	15	max	0	1	.008	2	0	9	-1.09e-7	10	NC	1	NC	1
220		min	0	3	-.006	3	0	3	-2.595e-4	3	NC	1	NC	1
221	16	max	0	1	.006	2	0	9	-1.09e-7	10	NC	1	NC	1
222		min	0	3	-.005	3	0	3	-2.595e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.004	2	0	9	-1.09e-7	10	NC	1	NC	1
224			min	0	3	-.003	3	0	3	-2.595e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	9	-1.09e-7	10	NC	1	NC	1
226			min	0	3	-.002	3	0	3	-2.595e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.09e-7	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.595e-4	3	NC	1	NC	1
229	M10	1	max	.002	2	.009	2	0	10	2.882e-4	1	NC	3	NC	1
230			min	-.003	3	-.009	3	-.002	1	-5.682e-4	3	4192.19	2	NC	1
231		2	max	.002	2	.009	2	0	3	2.741e-4	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.002	1	-5.487e-4	3	4575.147	2	NC	1
233		3	max	.002	2	.008	2	0	3	2.601e-4	1	NC	3	NC	1
234			min	-.003	3	-.009	3	-.001	1	-5.293e-4	3	5030.577	2	NC	1
235		4	max	.002	2	.007	2	0	3	2.46e-4	1	NC	1	NC	1
236			min	-.002	3	-.008	3	-.001	1	-5.098e-4	3	5575.961	2	NC	1
237		5	max	.002	2	.006	2	0	3	2.319e-4	1	NC	1	NC	1
238			min	-.002	3	-.008	3	-.001	1	-4.904e-4	3	6234.593	2	NC	1
239		6	max	.001	2	.006	2	0	3	2.178e-4	1	NC	1	NC	1
240			min	-.002	3	-.008	3	-.001	1	-4.709e-4	3	7038.023	2	NC	1
241		7	max	.001	2	.005	2	0	3	2.037e-4	1	NC	1	NC	1
242			min	-.002	3	-.007	3	-.001	1	-4.515e-4	3	8029.804	2	NC	1
243		8	max	.001	2	.004	2	0	3	1.897e-4	1	NC	1	NC	1
244			min	-.002	3	-.007	3	0	1	-4.32e-4	3	9271.395	2	NC	1
245		9	max	.001	2	.004	2	0	3	1.756e-4	1	NC	1	NC	1
246			min	-.002	3	-.006	3	0	1	-4.126e-4	3	NC	1	NC	1
247		10	max	0	2	.003	2	0	3	1.615e-4	1	NC	1	NC	1
248			min	-.001	3	-.006	3	0	1	-3.931e-4	3	NC	1	NC	1
249		11	max	0	2	.003	2	0	3	1.474e-4	1	NC	1	NC	1
250			min	-.001	3	-.005	3	0	1	-3.737e-4	3	NC	1	NC	1
251		12	max	0	2	.002	2	0	3	1.333e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-3.542e-4	3	NC	1	NC	1
253		13	max	0	2	.002	2	0	3	1.193e-4	1	NC	1	NC	1
254			min	0	3	-.004	3	0	1	-3.348e-4	3	NC	1	NC	1
255		14	max	0	2	.001	2	0	3	1.052e-4	1	NC	1	NC	1
256			min	0	3	-.003	3	0	1	-3.154e-4	3	NC	1	NC	1
257		15	max	0	2	0	2	0	3	9.111e-5	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.959e-4	3	NC	1	NC	1
259		16	max	0	2	0	2	0	3	7.703e-5	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-2.765e-4	3	NC	1	NC	1
261		17	max	0	2	0	2	0	3	6.295e-5	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-2.57e-4	3	NC	1	NC	1
263		18	max	0	2	0	2	0	3	4.888e-5	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-2.376e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	3.48e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-2.181e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	1.03e-4	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-1.657e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	7.842e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-2.644e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	1	5.387e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-3.631e-5	1	NC	1	NC	1
273		4	max	0	3	0	2	0	11	2.932e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-4.618e-5	1	NC	1	NC	1
275		5	max	0	3	0	2	0	11	4.779e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.606e-5	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	8.013e-7	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-6.593e-5	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	9.e-7	10	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-7.58e-5	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	9.987e-7	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-8.567e-5	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	1.097e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-9.554e-5	1	NC	1	NC	1
285		10	max	.001	3	.002	2	0	10	1.196e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.179e-4	3	NC	1	NC	1
287		11	max	.001	3	.002	2	0	10	1.295e-6	10	NC	1	NC	1
288			min	-.001	2	-.007	3	-.003	3	-1.425e-4	3	NC	1	NC	1
289		12	max	.001	3	.003	2	0	10	1.393e-6	10	NC	1	NC	1
290			min	-.001	2	-.008	3	-.003	3	-1.67e-4	3	NC	1	NC	1
291		13	max	.001	3	.003	2	0	10	1.492e-6	10	NC	1	NC	1
292			min	-.001	2	-.008	3	-.003	3	-1.916e-4	3	NC	1	NC	1
293		14	max	.001	3	.004	2	0	10	1.591e-6	10	NC	1	NC	1
294			min	-.002	2	-.008	3	-.003	3	-2.161e-4	3	NC	1	NC	1
295		15	max	.002	3	.005	2	0	10	1.69e-6	10	NC	1	NC	1
296			min	-.002	2	-.008	3	-.003	3	-2.407e-4	3	9088.772	2	NC	1
297		16	max	.002	3	.006	2	0	10	1.788e-6	10	NC	1	NC	1
298			min	-.002	2	-.008	3	-.003	3	-2.652e-4	3	7691.709	2	NC	1
299		17	max	.002	3	.007	2	0	10	1.887e-6	10	NC	1	NC	1
300			min	-.002	2	-.008	3	-.003	3	-2.898e-4	3	6612.603	2	NC	1
301		18	max	.002	3	.008	2	0	10	1.986e-6	10	NC	1	NC	1
302			min	-.002	2	-.008	3	-.003	1	-3.143e-4	3	5769.443	2	NC	1
303		19	max	.002	3	.009	2	0	10	2.084e-6	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.003	1	-3.389e-4	3	5104.66	2	NC	1
305	M12	1	max	.001	1	.011	2	.003	1	3.667e-4	3	NC	1	NC	2
306			min	0	12	-.009	3	0	10	-2.916e-6	10	NC	1	7728.053	1
307		2	max	.001	1	.01	2	.002	1	3.667e-4	3	NC	1	NC	2
308			min	0	12	-.009	3	0	10	-2.916e-6	10	NC	1	8428.991	1
309		3	max	.001	1	.01	2	.002	1	3.667e-4	3	NC	1	NC	2
310			min	0	12	-.008	3	0	10	-2.916e-6	10	NC	1	9263.296	1
311		4	max	.001	1	.009	2	.002	1	3.667e-4	3	NC	1	NC	1
312			min	0	12	-.008	3	0	10	-2.916e-6	10	NC	1	NC	1
313		5	max	.001	1	.008	2	.002	1	3.667e-4	3	NC	1	NC	1
314			min	0	12	-.007	3	0	10	-2.916e-6	10	NC	1	NC	1
315		6	max	0	1	.008	2	.001	1	3.667e-4	3	NC	1	NC	1
316			min	0	12	-.007	3	0	10	-2.916e-6	10	NC	1	NC	1
317		7	max	0	1	.007	2	.001	1	3.667e-4	3	NC	1	NC	1
318			min	0	12	-.006	3	0	10	-2.916e-6	10	NC	1	NC	1
319		8	max	0	1	.007	2	.001	1	3.667e-4	3	NC	1	NC	1
320			min	0	12	-.006	3	0	10	-2.916e-6	10	NC	1	NC	1
321		9	max	0	1	.006	2	0	1	3.667e-4	3	NC	1	NC	1
322			min	0	12	-.005	3	0	10	-2.916e-6	10	NC	1	NC	1
323		10	max	0	1	.005	2	0	1	3.667e-4	3	NC	1	NC	1
324			min	0	12	-.005	3	0	10	-2.916e-6	10	NC	1	NC	1
325		11	max	0	1	.005	2	0	1	3.667e-4	3	NC	1	NC	1
326			min	0	12	-.004	3	0	10	-2.916e-6	10	NC	1	NC	1
327		12	max	0	1	.004	2	0	1	3.667e-4	3	NC	1	NC	1
328			min	0	12	-.004	3	0	10	-2.916e-6	10	NC	1	NC	1
329		13	max	0	1	.004	2	0	1	3.667e-4	3	NC	1	NC	1
330			min	0	12	-.003	3	0	10	-2.916e-6	10	NC	1	NC	1
331		14	max	0	1	.003	2	0	1	3.667e-4	3	NC	1	NC	1
332			min	0	12	-.003	3	0	10	-2.916e-6	10	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	3.667e-4	3	NC	1	NC	1
334			min	0	12	-.002	3	0	10	-2.916e-6	10	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	3.667e-4	3	NC	1	NC	1
336			min	0	12	-.002	3	0	10	-2.916e-6	10	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.667e-4	3	NC	1	NC	1
338			min	0	12	-.001	3	0	10	-2.916e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.667e-4	3	NC	1	NC	1
340			min	0	12	0	3	0	10	-2.916e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.667e-4	3	NC	1	NC	1
342			min	0	1	0	1	0	1	-2.916e-6	10	NC	1	NC	1
343	M1	1	max	.009	3	.025	3	.004	3	5.928e-3	2	NC	1	NC	1
344			min	-.009	2	-.021	2	0	9	-8.687e-3	3	NC	1	NC	1
345		2	max	.008	3	.015	3	.003	3	2.923e-3	2	NC	4	NC	1
346			min	-.009	2	-.012	2	-.002	1	-4.279e-3	3	4707.979	3	NC	1
347		3	max	.008	3	.005	3	.002	3	4.653e-5	3	NC	4	NC	1
348			min	-.009	2	-.004	2	-.003	1	-1.467e-4	1	2438.764	3	NC	1
349		4	max	.008	3	.003	2	.002	3	4.786e-5	3	NC	4	NC	1
350			min	-.009	2	-.003	3	-.003	1	-1.227e-4	1	1737.732	3	NC	1
351		5	max	.008	3	.01	2	.001	3	4.92e-5	3	NC	4	NC	1
352			min	-.009	2	-.009	3	-.003	1	-9.88e-5	1	1403.461	3	NC	1
353		6	max	.008	3	.015	2	.001	3	5.053e-5	3	NC	4	NC	1
354			min	-.009	2	-.015	3	-.003	1	-7.487e-5	1	1216.919	3	NC	1
355		7	max	.008	3	.019	2	.001	3	5.187e-5	3	NC	4	NC	1
356			min	-.009	2	-.019	3	-.003	1	-5.172e-5	9	1106.057	2	NC	1
357		8	max	.008	3	.023	2	0	3	5.32e-5	3	NC	4	NC	1
358			min	-.009	2	-.022	3	-.002	1	-3.36e-5	9	1019.529	2	NC	1
359		9	max	.008	3	.025	2	0	3	5.454e-5	3	NC	4	NC	1
360			min	-.009	2	-.023	3	-.001	9	-1.549e-5	9	967.761	2	NC	1
361		10	max	.008	3	.026	2	0	3	5.588e-5	3	NC	4	NC	1
362			min	-.009	2	-.024	3	0	9	1.695e-7	10	943.043	2	NC	1
363		11	max	.008	3	.025	2	.001	3	5.721e-5	3	NC	4	NC	1
364			min	-.009	2	-.023	3	0	9	-1.965e-7	10	942.536	2	NC	1
365		12	max	.008	3	.024	2	.001	3	6.873e-5	1	NC	4	NC	1
366			min	-.009	2	-.021	3	0	10	-5.626e-7	10	967.225	2	NC	1
367		13	max	.008	3	.021	2	.002	1	9.266e-5	1	NC	4	NC	1
368			min	-.009	2	-.018	3	0	10	-9.286e-7	10	1022.495	2	NC	1
369		14	max	.008	3	.016	2	.002	1	1.166e-4	1	NC	4	NC	1
370			min	-.009	2	-.014	3	0	10	-1.295e-6	10	1120.886	2	NC	1
371		15	max	.008	3	.01	2	.002	1	1.405e-4	1	NC	4	NC	1
372			min	-.009	2	-.009	3	0	10	-1.661e-6	10	1290.039	2	NC	1
373		16	max	.008	3	.003	2	.002	1	1.577e-4	1	NC	4	NC	1
374			min	-.009	2	-.003	3	0	10	-1.931e-6	10	1597.846	2	NC	1
375		17	max	.008	3	.005	3	.002	1	6.868e-5	3	NC	4	NC	1
376			min	-.009	2	-.006	2	0	10	-9.393e-6	9	2263.728	2	NC	1
377		18	max	.008	3	.012	3	0	3	4.26e-3	2	NC	4	NC	1
378			min	-.009	2	-.017	2	0	10	-2.253e-3	3	4387.504	2	NC	1
379		19	max	.008	3	.021	3	0	3	8.59e-3	2	NC	1	NC	1
380			min	-.009	2	-.028	2	0	9	-4.619e-3	3	NC	1	NC	1
381	M5	1	max	.025	3	.078	3	.004	3	8.485e-6	3	NC	1	NC	1
382			min	-.028	2	-.065	2	0	9	0	15	NC	1	NC	1
383		2	max	.025	3	.046	3	.005	3	1.366e-4	3	NC	4	NC	1
384			min	-.028	2	-.038	2	0	9	-1.572e-5	9	1507.405	3	NC	1
385		3	max	.025	3	.016	3	.006	3	2.622e-4	3	NC	5	NC	1
386			min	-.028	2	-.012	2	0	9	-3.125e-5	9	781.279	3	NC	1
387		4	max	.025	3	.011	2	.007	3	2.534e-4	3	NC	5	NC	1
388			min	-.028	2	-.009	3	0	9	-2.977e-5	9	557.431	3	NC	1
389		5	max	.025	3	.031	2	.008	3	2.445e-4	3	NC	5	NC	1
390			min	-.028	2	-.03	3	0	9	-2.829e-5	9	450.822	3	9368.834	3
391		6	max	.025	3	.048	2	.008	3	2.357e-4	3	NC	5	NC	1
392			min	-.028	2	-.046	3	0	9	-2.681e-5	9	391.444	3	8457.521	3
393		7	max	.025	3	.061	2	.009	3	2.268e-4	3	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.028	2	-.059	3	0	9	-2.533e-5	9	348.751	2	8039.285	3
395		8	max	.025	3	.072	2	.008	3	2.179e-4	3	NC	5	NC	1
396			min	-.028	2	-.067	3	0	9	-2.385e-5	9	321.379	2	7949.458	3
397		9	max	.025	3	.078	2	.008	3	2.091e-4	3	NC	5	NC	1
398			min	-.028	2	-.072	3	0	9	-2.237e-5	9	304.993	2	8126.19	3
399		10	max	.025	3	.081	2	.008	3	2.002e-4	3	NC	5	NC	1
400			min	-.028	2	-.073	3	0	9	-2.089e-5	9	297.152	2	8562.681	3
401		11	max	.025	3	.08	2	.007	3	1.914e-4	3	NC	5	NC	1
402			min	-.028	2	-.071	3	0	9	-1.941e-5	9	296.957	2	9295.422	3
403		12	max	.025	3	.075	2	.006	3	1.825e-4	3	NC	5	NC	1
404			min	-.028	2	-.065	3	0	9	-1.793e-5	9	304.716	2	NC	1
405		13	max	.024	3	.065	2	.006	3	1.737e-4	3	NC	5	NC	1
406			min	-.028	2	-.056	3	0	9	-1.645e-5	9	322.123	2	NC	1
407		14	max	.024	3	.051	2	.005	3	1.648e-4	3	NC	5	NC	1
408			min	-.028	2	-.043	3	0	9	-1.497e-5	9	353.135	2	NC	1
409		15	max	.024	3	.033	2	.004	3	1.56e-4	3	NC	5	NC	1
410			min	-.028	2	-.027	3	0	9	-1.349e-5	9	406.47	2	NC	1
411		16	max	.024	3	.009	2	.003	3	1.424e-4	3	NC	5	NC	1
412			min	-.028	2	-.008	3	0	9	-1.312e-5	9	503.555	2	NC	1
413		17	max	.024	3	.014	3	.002	3	1.641e-5	3	NC	5	NC	1
414			min	-.028	2	-.02	2	0	9	-3.921e-5	9	713.745	2	NC	1
415		18	max	.024	3	.038	3	.001	3	6.398e-6	3	NC	4	NC	1
416			min	-.028	2	-.053	2	0	9	-2.006e-5	9	1383.807	2	NC	1
417		19	max	.024	3	.063	3	0	3	-3.484e-8	15	NC	1	NC	1
418			min	-.028	2	-.088	2	0	9	-1.426e-6	3	NC	1	NC	1
419	M9	1	max	.009	3	.024	3	.003	3	8.704e-3	3	NC	1	NC	1
420			min	-.009	2	-.021	2	-.001	9	-5.928e-3	2	NC	1	NC	1
421		2	max	.009	3	.014	3	.002	3	4.282e-3	3	NC	4	NC	1
422			min	-.009	2	-.012	2	0	10	-2.923e-3	2	4710.139	3	NC	1
423		3	max	.009	3	.005	3	.002	1	9.605e-5	1	NC	4	NC	1
424			min	-.009	2	-.004	2	0	10	-5.836e-5	3	2439.901	3	NC	1
425		4	max	.009	3	.003	2	.002	1	7.438e-5	1	NC	4	NC	1
426			min	-.009	2	-.003	3	-.001	3	-6.348e-5	3	1738.517	3	NC	1
427		5	max	.008	3	.01	2	.002	1	5.271e-5	1	NC	4	NC	1
428			min	-.009	2	-.01	3	-.002	3	-6.859e-5	3	1404.045	3	9126.892	3
429		6	max	.008	3	.015	2	.002	1	3.104e-5	1	NC	4	NC	1
430			min	-.009	2	-.015	3	-.003	3	-7.371e-5	3	1217.371	3	7938.936	3
431		7	max	.008	3	.019	2	.002	1	1.598e-5	11	NC	4	NC	1
432			min	-.009	2	-.019	3	-.004	3	-7.882e-5	3	1106.342	2	7253.483	3
433		8	max	.008	3	.023	2	.001	1	5.025e-6	11	NC	4	NC	1
434			min	-.009	2	-.022	3	-.005	3	-8.394e-5	3	1019.801	2	6874.277	3
435		9	max	.008	3	.025	2	0	11	-4.062e-7	10	NC	4	NC	1
436			min	-.009	2	-.024	3	-.005	3	-8.905e-5	3	968.029	2	6712.753	3
437		10	max	.008	3	.026	2	0	11	-4.821e-8	10	NC	4	NC	1
438			min	-.009	2	-.024	3	-.005	3	-9.416e-5	3	943.312	2	6731.318	3
439		11	max	.008	3	.025	2	0	10	3.098e-7	10	NC	4	NC	1
440			min	-.009	2	-.023	3	-.005	3	-9.928e-5	3	942.812	2	6923.731	3
441		12	max	.008	3	.024	2	0	10	6.678e-7	10	NC	4	NC	1
442			min	-.009	2	-.021	3	-.005	3	-1.044e-4	3	967.516	2	7311.107	3
443		13	max	.008	3	.021	2	0	10	1.026e-6	10	NC	4	NC	1
444			min	-.009	2	-.018	3	-.004	3	-1.207e-4	1	1022.808	2	7949.442	3
445		14	max	.008	3	.016	2	0	10	1.384e-6	10	NC	4	NC	1
446			min	-.009	2	-.014	3	-.004	3	-1.423e-4	1	1121.234	2	8954.572	3
447		15	max	.008	3	.01	2	0	10	1.742e-6	10	NC	4	NC	1
448			min	-.009	2	-.009	3	-.003	3	-1.64e-4	1	1290.443	2	NC	1
449		16	max	.008	3	.003	2	0	10	1.996e-6	10	NC	4	NC	1
450			min	-.009	2	-.003	3	-.003	1	-1.803e-4	1	1598.341	2	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.008	3	.005	3	0	10	8.247e-5	3	NC	4	NC	1
452			min	-.009	2	-.006	2	-.003	1	-6.921e-5	9	2264.38	2	NC	1
453		18	max	.008	3	.012	3	0	10	2.33e-3	3	NC	4	NC	1
454			min	-.009	2	-.017	2	-.002	1	-4.26e-3	2	4388.727	2	NC	1
455		19	max	.008	3	.021	3	0	3	4.616e-3	3	NC	1	NC	1
456			min	-.009	2	-.028	2	0	9	-8.591e-3	2	NC	1	NC	1
457	M13	1	max	.001	9	.024	3	.009	3	3.839e-3	3	NC	1	NC	1
458			min	-.003	3	-.021	2	-.009	2	-3.29e-3	2	NC	1	NC	1
459		2	max	.001	9	.069	3	.007	3	4.753e-3	3	NC	4	NC	1
460			min	-.003	3	-.051	2	-.008	2	-4.08e-3	2	2138.174	3	NC	1
461		3	max	.001	9	.107	3	.008	9	5.667e-3	3	NC	4	NC	2
462			min	-.003	3	-.077	2	-.007	2	-4.871e-3	2	1160.617	3	9077.61	1
463		4	max	0	9	.133	3	.011	9	6.582e-3	3	NC	5	NC	2
464			min	-.003	3	-.096	2	-.007	2	-5.662e-3	2	883.822	3	6594.261	1
465		5	max	0	9	.145	3	.013	9	7.496e-3	3	NC	5	NC	2
466			min	-.003	3	-.105	2	-.009	2	-6.452e-3	2	798.415	3	6257.03	1
467		6	max	0	9	.142	3	.013	3	8.41e-3	3	NC	5	NC	2
468			min	-.003	3	-.104	2	-.013	2	-7.243e-3	2	815.946	3	7735.987	1
469		7	max	0	9	.128	3	.016	3	9.324e-3	3	NC	5	NC	1
470			min	-.004	3	-.096	2	-.017	2	-8.034e-3	2	928.352	3	NC	1
471		8	max	0	9	.107	3	.019	3	1.024e-2	3	NC	4	NC	1
472			min	-.004	3	-.084	2	-.022	2	-8.825e-3	2	1163.675	3	7206.093	2
473		9	max	0	9	.087	3	.022	3	1.115e-2	3	NC	4	NC	1
474			min	-.004	3	-.071	2	-.026	2	-9.615e-3	2	1539.126	3	5497.748	2
475		10	max	0	9	.078	3	.025	3	1.207e-2	3	NC	4	NC	4
476			min	-.004	3	-.065	2	-.028	2	-1.041e-2	2	1813.155	3	4983.909	2
477		11	max	0	9	.087	3	.027	3	1.116e-2	3	NC	4	NC	1
478			min	-.004	3	-.071	2	-.026	2	-9.615e-3	2	1539.125	3	5177.917	3
479		12	max	0	9	.107	3	.027	3	1.024e-2	3	NC	4	NC	1
480			min	-.004	3	-.084	2	-.022	2	-8.825e-3	2	1163.673	3	5079.545	3
481		13	max	0	9	.128	3	.026	3	9.332e-3	3	NC	5	NC	1
482			min	-.004	3	-.096	2	-.017	2	-8.034e-3	2	928.351	3	5360.234	3
483		14	max	0	9	.142	3	.024	3	8.42e-3	3	NC	5	NC	2
484			min	-.004	3	-.104	2	-.013	2	-7.243e-3	2	815.945	3	6079.24	3
485		15	max	0	9	.145	3	.021	3	7.508e-3	3	NC	5	NC	2
486			min	-.004	3	-.105	2	-.009	2	-6.453e-3	2	798.414	3	6260.521	1
487		16	max	0	9	.134	3	.018	3	6.596e-3	3	NC	5	NC	2
488			min	-.004	3	-.096	2	-.007	2	-5.662e-3	2	883.821	3	6603.931	1
489		17	max	0	9	.108	3	.014	3	5.684e-3	3	NC	4	NC	2
490			min	-.004	3	-.077	2	-.007	2	-4.871e-3	2	1160.616	3	9100.991	1
491		18	max	0	9	.07	3	.011	3	4.773e-3	3	NC	4	NC	1
492			min	-.004	3	-.051	2	-.008	2	-4.081e-3	2	2138.172	3	NC	1
493		19	max	0	9	.025	3	.009	3	3.861e-3	3	NC	1	NC	1
494			min	-.004	3	-.021	2	-.009	2	-3.29e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.021	3	.008	3	4.195e-3	2	NC	1	NC	1
496			min	0	3	-.028	2	-.009	2	-3.14e-3	3	NC	1	NC	1
497		2	max	0	9	.045	3	.011	3	5.207e-3	2	NC	4	NC	1
498			min	0	3	-.072	2	-.008	2	-3.853e-3	3	2162.003	2	NC	1
499		3	max	0	9	.065	3	.014	3	6.219e-3	2	NC	4	NC	2
500			min	0	3	-.11	2	-.007	2	-4.566e-3	3	1170.537	2	9114.508	1
501		4	max	0	9	.08	3	.017	3	7.232e-3	2	NC	5	NC	2
502			min	0	3	-.136	2	-.007	2	-5.279e-3	3	887.367	2	6621.846	1
503		5	max	0	9	.088	3	.02	3	8.244e-3	2	NC	5	NC	2
504			min	0	3	-.148	2	-.009	2	-5.993e-3	3	795.921	2	6288.237	1
505		6	max	0	9	.089	3	.023	3	9.257e-3	2	NC	5	NC	2
506			min	0	3	-.147	2	-.013	2	-6.706e-3	3	804.402	2	6678.701	3
507		7	max	0	9	.084	3	.024	3	1.027e-2	2	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.134	2	-.017	2	-7.419e-3	3	899.273	2	5973.177	3
509	8	max	0	9	.076	3	.025	3	1.128e-2	2	NC	4	NC	1
510		min	0	3	-.115	2	-.022	2	-8.132e-3	3	1096.264	2	5661.608	3
511	9	max	0	9	.067	3	.025	3	1.229e-2	2	NC	4	NC	1
512		min	0	3	-.096	2	-.026	2	-8.845e-3	3	1394.056	2	5522.028	2
513	10	max	0	9	.063	3	.024	3	1.331e-2	2	NC	4	NC	4
514		min	0	3	-.088	2	-.028	2	-9.559e-3	3	1598.774	2	5004.549	2
515	11	max	0	9	.067	3	.023	3	1.229e-2	2	NC	4	NC	1
516		min	0	3	-.096	2	-.026	2	-8.843e-3	3	1394.056	2	5522.035	2
517	12	max	0	9	.076	3	.021	3	1.128e-2	2	NC	4	NC	1
518		min	0	3	-.115	2	-.022	2	-8.128e-3	3	1096.264	2	7243.991	2
519	13	max	0	9	.084	3	.02	3	1.027e-2	2	NC	5	NC	1
520		min	0	3	-.134	2	-.017	2	-7.412e-3	3	899.273	2	8404.51	3
521	14	max	0	9	.089	3	.018	3	9.257e-3	2	NC	5	NC	2
522		min	0	3	-.147	2	-.013	2	-6.697e-3	3	804.402	2	7799.7	1
523	15	max	0	9	.088	3	.015	3	8.245e-3	2	NC	5	NC	2
524		min	0	3	-.148	2	-.009	2	-5.981e-3	3	795.921	2	6299.886	1
525	16	max	0	9	.08	3	.013	3	7.233e-3	2	NC	5	NC	2
526		min	0	3	-.136	2	-.007	2	-5.266e-3	3	887.367	2	6640.159	1
527	17	max	0	9	.065	3	.011	3	6.221e-3	2	NC	4	NC	2
528		min	0	3	-.11	2	-.007	2	-4.55e-3	3	1170.537	2	9151.375	1
529	18	max	0	9	.045	3	.009	3	5.208e-3	2	NC	4	NC	1
530		min	0	3	-.072	2	-.008	2	-3.835e-3	3	2162.003	2	NC	1
531	19	max	0	9	.021	3	.008	3	4.196e-3	2	NC	1	NC	1
532		min	0	3	-.028	2	-.009	2	-3.119e-3	3	NC	1	NC	1
533	M15	1	max	0	1	0	0	1	4.042e-4	3	NC	1	NC	1
534		min	0	1	0	0	0	1	-4.892e-5	2	NC	1	NC	1
535	2	max	0	3	0	15	0	1	8.124e-4	3	NC	1	NC	1
536		min	0	2	-.003	4	0	3	-4.478e-4	2	NC	1	NC	1
537	3	max	0	3	-.001	15	.003	1	1.221e-3	3	NC	1	NC	1
538		min	0	2	-.006	4	-.003	3	-8.467e-4	2	NC	1	9266.42	3
539	4	max	0	3	-.002	15	.006	2	1.629e-3	3	NC	1	NC	4
540		min	0	2	-.008	4	-.007	3	-1.246e-3	2	7821.324	4	5136.506	3
541	5	max	0	3	-.002	15	.009	2	2.037e-3	3	NC	3	NC	4
542		min	0	2	-.01	4	-.012	3	-1.645e-3	2	6103.059	4	3381.863	3
543	6	max	0	3	-.003	15	.013	2	2.445e-3	3	NC	5	NC	4
544		min	-.001	2	-.012	4	-.017	3	-2.043e-3	2	5136.369	4	2467.774	3
545	7	max	0	3	-.003	15	.017	2	2.853e-3	3	NC	5	NC	4
546		min	-.001	2	-.014	4	-.022	3	-2.442e-3	2	4555.033	4	1932.194	3
547	8	max	0	3	-.004	15	.021	2	3.261e-3	3	NC	5	NC	4
548		min	-.002	2	-.015	4	-.027	3	-2.841e-3	2	4206.143	4	1595.048	3
549	9	max	0	3	-.004	15	.025	2	3.67e-3	3	NC	5	NC	4
550		min	-.002	2	-.016	4	-.032	3	-3.24e-3	2	4018.351	4	1374.227	3
551	10	max	0	3	-.004	15	.028	2	4.078e-3	3	NC	5	NC	4
552		min	-.002	2	-.016	4	-.035	3	-3.639e-3	2	3958.942	4	1228.431	3
553	11	max	.001	3	-.004	15	.029	2	4.486e-3	3	NC	5	NC	4
554		min	-.002	2	-.016	4	-.038	3	-4.038e-3	2	4018.351	4	1135.961	3
555	12	max	.001	3	-.004	15	.03	2	4.894e-3	3	NC	5	NC	4
556		min	-.002	2	-.015	4	-.038	3	-4.437e-3	2	4206.143	4	1086.067	3
557	13	max	.001	3	-.003	15	.029	2	5.302e-3	3	NC	5	NC	4
558		min	-.003	2	-.014	4	-.037	3	-4.836e-3	2	4555.033	4	1075.676	3
559	14	max	.001	3	-.003	15	.025	2	5.71e-3	3	NC	5	NC	4
560		min	-.003	2	-.012	4	-.034	3	-5.235e-3	2	5136.369	4	1109.544	3
561	15	max	.001	3	-.002	15	.021	1	6.119e-3	3	NC	3	NC	4
562		min	-.003	2	-.011	4	-.028	3	-5.634e-3	2	6103.059	4	1204.964	3
563	16	max	.002	3	0	2	.014	1	6.527e-3	3	NC	1	NC	4
564		min	-.003	2	-.008	4	-.019	3	-6.033e-3	2	7821.324	4	1408.825	3

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	.002	3	.002	2	.006	1	6.935e-3	3	NC	1	NC	4
566			min	-.003	2	-.006	4	-.006	3	-6.431e-3	2	NC	1	1868.178	3
567		18	max	.002	3	.004	2	.01	3	7.343e-3	3	NC	1	NC	4
568			min	-.004	2	-.003	4	-.012	2	-6.83e-3	2	NC	1	3326.847	3
569		19	max	.002	3	.007	2	.031	3	7.751e-3	3	NC	1	NC	1
570			min	-.004	2	-.001	9	-.03	2	-7.229e-3	2	NC	1	NC	1
571	M16A	1	max	0	2	.002	2	.009	3	2.206e-3	3	NC	1	NC	1
572			min	-.002	3	-.002	3	-.009	2	-2.242e-3	2	NC	1	NC	1
573		2	max	0	2	0	2	.002	3	2.124e-3	3	NC	1	NC	1
574			min	-.002	3	-.003	4	-.004	2	-2.14e-3	2	NC	1	9285.847	3
575		3	max	0	2	-.001	15	.003	1	2.041e-3	3	NC	1	NC	4
576			min	-.002	3	-.006	4	-.003	3	-2.039e-3	2	NC	1	5259.305	3
577		4	max	0	2	-.002	15	.006	1	1.959e-3	3	NC	1	NC	4
578			min	-.002	3	-.008	4	-.008	3	-1.937e-3	2	7821.324	4	4004.762	3
579		5	max	0	2	-.002	15	.008	1	1.877e-3	3	NC	3	NC	4
580			min	-.001	3	-.011	4	-.011	3	-1.836e-3	2	6103.059	4	3463.339	3
581		6	max	0	2	-.003	15	.009	1	1.794e-3	3	NC	5	NC	4
582			min	-.001	3	-.012	4	-.012	3	-1.734e-3	2	5136.369	4	3229.926	3
583		7	max	0	2	-.003	15	.01	1	1.712e-3	3	NC	5	NC	4
584			min	-.001	3	-.014	4	-.013	3	-1.633e-3	2	4555.033	4	3178.056	3
585		8	max	0	2	-.004	15	.01	1	1.629e-3	3	NC	5	NC	4
586			min	-.001	3	-.015	4	-.013	3	-1.531e-3	2	4206.143	4	3265.246	3
587		9	max	0	2	-.004	15	.009	1	1.547e-3	3	NC	5	NC	4
588			min	-.001	3	-.016	4	-.013	3	-1.43e-3	2	4018.351	4	3487.29	3
589		10	max	0	2	-.004	15	.008	1	1.465e-3	3	NC	5	NC	4
590			min	0	3	-.016	4	-.011	3	-1.328e-3	2	3958.942	4	3868.246	3
591		11	max	0	2	-.004	15	.007	1	1.382e-3	3	NC	5	NC	4
592			min	0	3	-.016	4	-.01	3	-1.227e-3	2	4018.351	4	4466.45	3
593		12	max	0	2	-.004	15	.006	1	1.3e-3	3	NC	5	NC	4
594			min	0	3	-.015	4	-.008	3	-1.125e-3	2	4206.143	4	5398.339	3
595		13	max	0	2	-.003	15	.005	1	1.217e-3	3	NC	5	NC	2
596			min	0	3	-.014	4	-.006	3	-1.024e-3	2	4555.033	4	6900.14	3
597		14	max	0	2	-.003	15	.003	1	1.135e-3	3	NC	5	NC	1
598			min	0	3	-.012	4	-.004	3	-9.222e-4	2	5136.369	4	9497.023	3
599		15	max	0	2	-.002	15	.002	1	1.053e-3	3	NC	3	NC	1
600			min	0	3	-.01	4	-.002	3	-8.207e-4	2	6103.059	4	NC	1
601		16	max	0	2	-.002	15	.001	14	9.703e-4	3	NC	1	NC	1
602			min	0	3	-.008	4	0	3	-7.192e-4	2	7821.324	4	NC	1
603		17	max	0	2	-.001	15	0	4	8.879e-4	3	NC	1	NC	1
604			min	0	3	-.005	4	0	2	-6.177e-4	2	NC	1	NC	1
605		18	max	0	2	0	15	0	3	8.055e-4	3	NC	1	NC	1
606			min	0	3	-.003	4	0	2	-5.161e-4	2	NC	1	NC	1
607		19	max	0	1	0	1	0	1	7.231e-4	3	NC	1	NC	1
608			min	0	1	0	1	0	1	-4.146e-4	2	NC	1	NC	1



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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

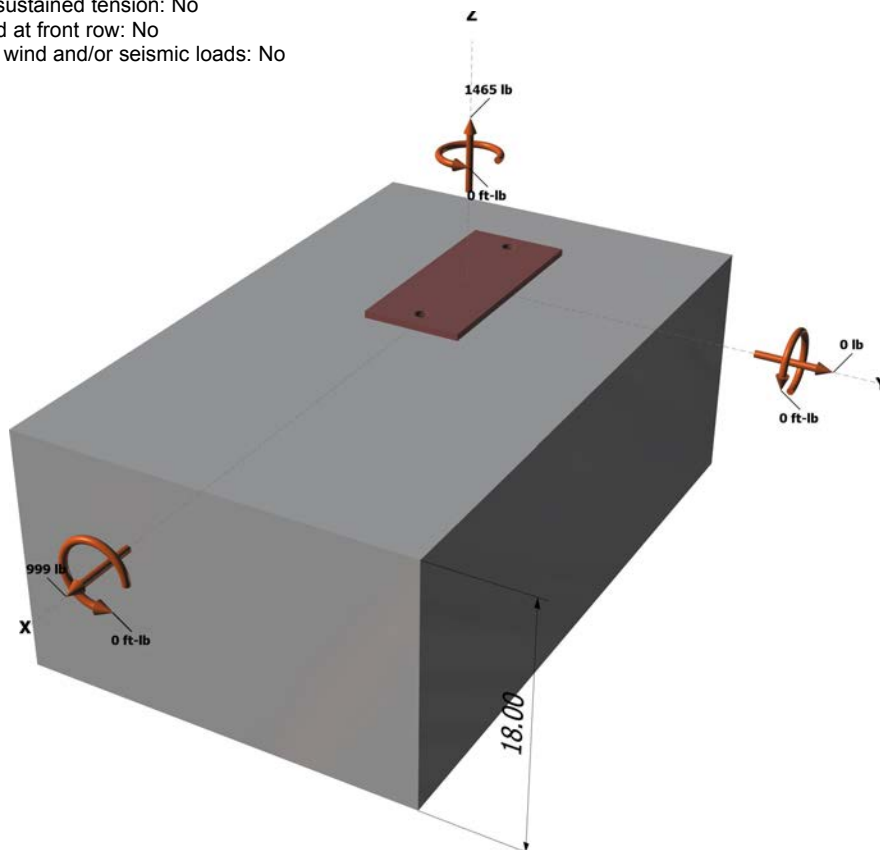
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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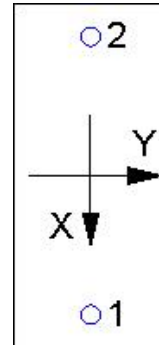
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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e'<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e'<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e'<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e'<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpq}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.