



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

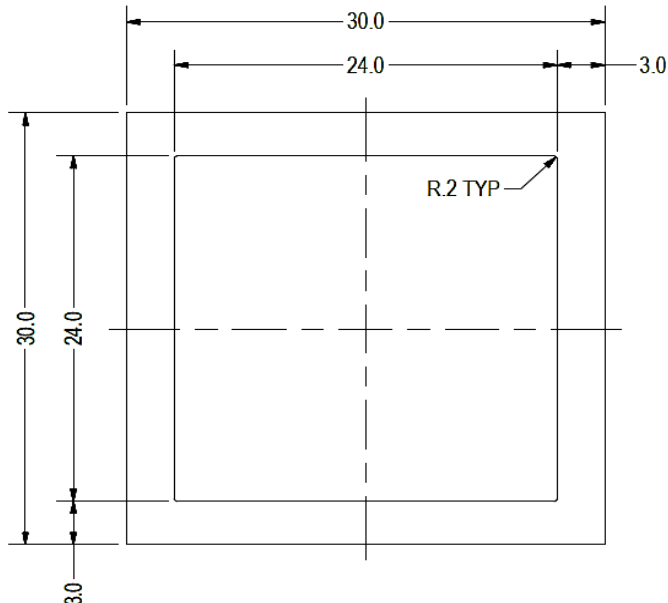
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.025 k-ft
$P_n$ =	0.127 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.736 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>19%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.599 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<b>13%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.175 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

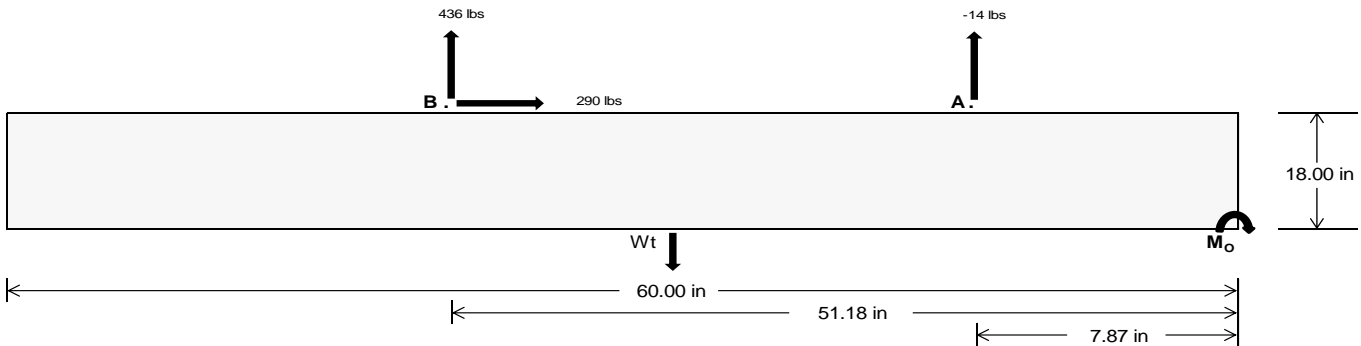
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>1.36</b>	<b>1813.74</b>	k
Compressive Load =	<b>821.48</b>	<b>1176.88</b>	k
Lateral Load =	<b>20.45</b>	<b>1206.62</b>	k
Moment (Weak Axis) =	<b>0.03</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 27403.4$  in-lbs  
Resisting Force Required = 913.45 lbs  
S.F. = 1.67  
Weight Required = 1522.41 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 289.99 lbs  
Friction = 0.4  
Weight Required = 724.97 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 289.99 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	275 lbs	275 lbs	275 lbs	275 lbs	326 lbs	326 lbs	326 lbs	326 lbs	422 lbs	422 lbs	422 lbs	422 lbs	29 lbs	29 lbs	29 lbs	29 lbs
$F_B$	175 lbs	175 lbs	175 lbs	175 lbs	532 lbs	532 lbs	532 lbs	532 lbs	512 lbs	512 lbs	512 lbs	512 lbs	-871 lbs	-871 lbs	-871 lbs	-871 lbs
$F_V$	25 lbs	25 lbs	25 lbs	25 lbs	521 lbs	521 lbs	521 lbs	521 lbs	407 lbs	407 lbs	407 lbs	407 lbs	-580 lbs	-580 lbs	-580 lbs	-580 lbs
$P_{total}$	2353 lbs	2444 lbs	2534 lbs	2625 lbs	2761 lbs	2852 lbs	2942 lbs	3033 lbs	2837 lbs	2927 lbs	3018 lbs	3109 lbs	299 lbs	354 lbs	408 lbs	463 lbs
$M$	236 lbs-ft	236 lbs-ft	236 lbs-ft	236 lbs-ft	443 lbs-ft	443 lbs-ft	443 lbs-ft	443 lbs-ft	486 lbs-ft	486 lbs-ft	486 lbs-ft	486 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft
$e$	0.10 ft	0.10 ft	0.09 ft	0.09 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	2.41 ft	2.04 ft	1.77 ft	1.56 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	236.6 psf	235.7 psf	234.9 psf	234.2 psf	254.8 psf	253.1 psf	251.6 psf	250.2 psf	257.5 psf	255.7 psf	254.0 psf	252.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	301.2 psf	297.4 psf	294.0 psf	290.8 psf	376.3 psf	369.0 psf	362.5 psf	356.4 psf	390.9 psf	383.0 psf	375.8 psf	369.2 psf	1247.6 psf	278.7 psf	193.6 psf	163.9 psf

Maximum Bearing Pressure = 1248 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

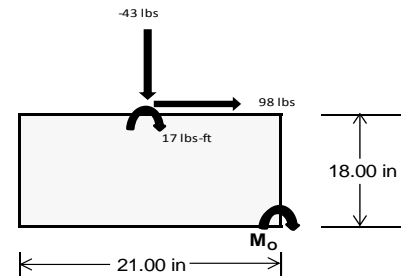
### Overturning Check

$M_o = 202.5 \text{ ft-lbs}$   
 Resisting Force Required = 231.44 lbs  
 S.F. = 1.67  
 Weight Required = 385.73 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	122 lbs	40 lbs	55 lbs	195 lbs	320 lbs	144 lbs	86 lbs	-43 lbs	21 lbs
$F_v$	12 lbs	98 lbs	12 lbs	9 lbs	74 lbs	9 lbs	12 lbs	98 lbs	12 lbs
$P_{total}$	2478 lbs	2396 lbs	2411 lbs	2438 lbs	2563 lbs	2387 lbs	774 lbs	646 lbs	710 lbs
$M$	33 lbs-ft	165 lbs-ft	34 lbs-ft	24 lbs-ft	123 lbs-ft	27 lbs-ft	33 lbs-ft	165 lbs-ft	34 lbs-ft
$e$	0.01 ft	0.07 ft	0.01 ft	0.01 ft	0.05 ft	0.01 ft	0.04 ft	0.25 ft	0.05 ft
$L/6$	0.29 ft	1.61 ft	1.72 ft	1.73 ft	1.65 ft	1.73 ft	1.66 ft	1.24 ft	1.65 ft
$f_{min}$	270.2 sqft	209.3 sqft	262.2 sqft	269.2 sqft	244.7 sqft	262.3 sqft	75.5 sqft	9.3 sqft	67.9 sqft
$f_{max}$	296.2 psf	338.4 psf	288.9 psf	288.0 psf	341.1 psf	283.3 psf	101.6 psf	138.3 psf	94.4 psf



Maximum Bearing Pressure = 341 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

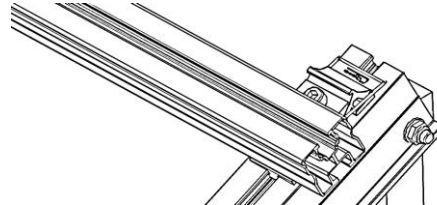
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.819 k
Allowable Uplift =	1.214 k
Utilization =	<u>67%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.101 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.632 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>

#### Diagonal Strut

Maximum Axial Load =	0.736 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



#### Rear Strut

Maximum Axial Load =	1.082 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.175 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	33.11 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.662 in
Max Drift, $\Delta_{MAX}$ =	0.051 in
	<u>0.051 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$117.177$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 45.00 \text{ in}$$

$$J = 0.255$$

$$121.682$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.9 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.271 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.28 \\ &21.7681 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.7 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.28 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\ \phi F_L &= 29.7 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.457 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.0$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

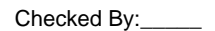
### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0      \.....\PVMini 60 Cell 1V 35° 120mph 30psf 3.75ft 7-05Pad 20





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	195.486	2	-.031	15	.076	1	0	10	0	4	0	15
30		min	-368.205	3	-.128	4	-.62	5	0	4	0	3	0	6
31	16	max	195.621	2	-.044	15	.076	1	0	10	0	4	0	15
32		min	-368.104	3	-.186	4	-.743	5	0	4	0	3	0	6
33	17	max	195.756	2	-.058	15	.076	1	0	10	0	4	0	15
34		min	-368.003	3	-.243	4	-.866	5	0	4	0	3	0	6
35	18	max	195.891	2	-.071	15	.076	1	0	10	0	9	0	15
36		min	-367.902	3	-.301	4	-.989	5	0	4	0	3	0	6
37	19	max	196.026	2	-.085	15	.076	1	0	10	0	9	0	15
38		min	-367.801	3	-.358	4	-1.112	5	0	4	0	3	0	6
39	M3	1	max	238.902	2	1.734	.007	10	0	5	0	4	0	6
40		min	-224.453	3	.407	15	-1.314	4	0	1	0	10	0	15
41	2	max	238.832	2	1.558	6	.007	10	0	5	0	1	0	2
42		min	-224.506	3	.365	15	-1.18	4	0	1	0	10	0	3
43	3	max	238.762	2	1.382	6	.007	10	0	5	0	1	0	2
44		min	-224.558	3	.324	15	-1.047	4	0	1	0	5	0	3
45	4	max	238.692	2	1.205	6	.007	10	0	5	0	1	0	15
46		min	-224.611	3	.283	15	-.913	4	0	1	0	5	0	4
47	5	max	238.622	2	1.029	6	.007	10	0	5	0	1	0	15
48		min	-224.663	3	.241	15	-.779	4	0	1	0	5	0	4
49	6	max	238.552	2	.852	6	.007	10	0	5	0	1	0	15
50		min	-224.716	3	.2	15	-.646	4	0	1	0	5	0	4
51	7	max	238.482	2	.676	6	.007	10	0	5	0	1	0	15
52		min	-224.768	3	.158	15	-.512	4	0	1	0	5	0	4
53	8	max	238.412	2	.5	6	.007	10	0	5	0	1	0	15
54		min	-224.821	3	.117	15	-.378	4	0	1	0	5	-.001	4
55	9	max	238.342	2	.323	6	.007	10	0	5	0	1	0	15
56		min	-224.873	3	.075	15	-.245	4	0	1	0	5	-.001	4
57	10	max	238.272	2	.147	6	.007	10	0	5	0	1	0	15
58		min	-224.926	3	.034	15	-.114	1	0	1	0	5	-.001	4
59	11	max	238.202	2	.006	2	.052	5	0	5	0	1	0	15
60		min	-224.978	3	-.054	3	-.114	1	0	1	0	5	-.001	4
61	12	max	238.132	2	-.049	15	.186	5	0	5	0	1	0	15
62		min	-225.031	3	-.206	4	-.114	1	0	1	0	5	-.001	4
63	13	max	238.062	2	-.091	15	.32	5	0	5	0	1	0	15
64		min	-225.083	3	-.382	4	-.114	1	0	1	0	5	-.001	4
65	14	max	237.992	2	-.132	15	.453	5	0	5	0	1	0	15
66		min	-225.136	3	-.559	4	-.114	1	0	1	0	5	-.001	4
67	15	max	237.922	2	-.173	15	.587	5	0	5	0	1	0	15
68		min	-225.188	3	-.735	4	-.114	1	0	1	0	5	0	4
69	16	max	237.852	2	-.215	15	.721	5	0	5	0	9	0	15
70		min	-225.241	3	-.911	4	-.114	1	0	1	0	5	0	4
71	17	max	237.782	2	-.256	15	.854	5	0	5	0	10	0	15
72		min	-225.293	3	-1.088	4	-.114	1	0	1	0	4	0	4
73	18	max	237.712	2	-.298	15	.988	5	0	5	0	10	0	15
74		min	-225.346	3	-1.264	4	-.114	1	0	1	0	4	0	4
75	19	max	237.642	2	-.339	15	1.122	5	0	5	0	5	0	1
76		min	-225.398	3	-1.441	4	-.114	1	0	1	0	1	0	1
77	M4	1	max	230.879	1	0	.043	10	0	1	0	5	0	1
78		min	20.758	15	0	1	-14.578	4	0	1	0	2	0	1
79	2	max	230.944	1	0	1	.043	10	0	1	0	10	0	1
80		min	20.777	15	0	1	-14.634	4	0	1	-.001	4	0	1
81	3	max	231.008	1	0	1	.043	10	0	1	0	10	0	1
82		min	20.797	15	0	1	-14.69	4	0	1	-.003	4	0	1
83	4	max	231.073	1	0	1	.043	10	0	1	0	10	0	1
84		min	20.816	15	0	1	-14.746	4	0	1	-.004	4	0	1
85	5	max	231.138	1	0	1	.043	10	0	1	0	10	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	20.836	15	0	1	-14.802	4	0	1	-.005	4	0	1
87		6	max	231.203	1	0	1	.043	10	0	1	0	10	0	1
88			min	20.855	15	0	1	-14.858	4	0	1	-.007	4	0	1
89		7	max	231.267	1	0	1	.043	10	0	1	0	10	0	1
90			min	20.875	15	0	1	-14.915	4	0	1	-.008	4	0	1
91		8	max	231.332	1	0	1	.043	10	0	1	0	10	0	1
92			min	20.894	15	0	1	-14.971	4	0	1	-.009	4	0	1
93		9	max	231.397	1	0	1	.043	10	0	1	0	10	0	1
94			min	20.914	15	0	1	-15.027	4	0	1	-.011	4	0	1
95		10	max	231.461	1	0	1	.043	10	0	1	0	10	0	1
96			min	20.933	15	0	1	-15.083	4	0	1	-.012	4	0	1
97		11	max	231.526	1	0	1	.043	10	0	1	0	10	0	1
98			min	20.953	15	0	1	-15.139	4	0	1	-.013	4	0	1
99		12	max	231.591	1	0	1	.043	10	0	1	0	10	0	1
100			min	20.972	15	0	1	-15.195	4	0	1	-.015	4	0	1
101		13	max	231.656	1	0	1	.043	10	0	1	0	10	0	1
102			min	20.992	15	0	1	-15.251	4	0	1	-.016	4	0	1
103		14	max	231.72	1	0	1	.043	10	0	1	0	10	0	1
104			min	21.012	15	0	1	-15.307	4	0	1	-.017	4	0	1
105		15	max	231.785	1	0	1	.043	10	0	1	0	10	0	1
106			min	21.031	15	0	1	-15.363	4	0	1	-.019	4	0	1
107		16	max	231.85	1	0	1	.043	10	0	1	0	10	0	1
108			min	21.051	15	0	1	-15.419	4	0	1	-.02	4	0	1
109		17	max	231.914	1	0	1	.043	10	0	1	0	10	0	1
110			min	21.07	15	0	1	-15.475	4	0	1	-.021	4	0	1
111		18	max	231.979	1	0	1	.043	10	0	1	0	10	0	1
112			min	21.09	15	0	1	-15.531	4	0	1	-.023	4	0	1
113		19	max	232.044	1	0	1	.043	10	0	1	0	10	0	1
114			min	21.109	15	0	1	-15.587	4	0	1	-.024	4	0	1
115	M6	1	max	596.189	2	.658	6	1.064	4	0	3	0	3	0	1
116			min	-1081.933	3	.145	15	-.288	3	0	5	0	2	0	1
117		2	max	596.324	2	.601	6	.941	4	0	3	0	3	0	15
118			min	-1081.832	3	.131	15	-.288	3	0	5	0	2	0	6
119		3	max	596.459	2	.543	6	.817	4	0	3	0	4	0	15
120			min	-1081.731	3	.118	15	-.288	3	0	5	0	2	0	6
121		4	max	596.594	2	.486	2	.694	4	0	3	0	4	0	15
122			min	-1081.63	3	.104	15	-.288	3	0	5	0	2	0	6
123		5	max	596.729	2	.442	2	.571	4	0	3	0	4	0	15
124			min	-1081.529	3	.091	15	-.288	3	0	5	0	2	0	6
125		6	max	596.864	2	.397	2	.448	4	0	3	0	4	0	15
126			min	-1081.428	3	.075	12	-.288	3	0	5	0	1	0	6
127		7	max	596.999	2	.352	2	.325	4	0	3	0	4	0	15
128			min	-1081.326	3	.053	12	-.288	3	0	5	0	1	0	2
129		8	max	597.134	2	.307	2	.202	4	0	3	0	4	0	15
130			min	-1081.225	3	.03	12	-.288	3	0	5	0	3	0	2
131		9	max	597.268	2	.262	2	.078	4	0	3	0	4	0	15
132			min	-1081.124	3	.004	3	-.288	3	0	5	0	3	0	2
133		10	max	597.403	2	.218	2	.014	9	0	3	0	4	0	15
134			min	-1081.023	3	-.029	3	-.288	3	0	5	0	3	0	2
135		11	max	597.538	2	.173	2	.014	9	0	3	0	4	0	12
136			min	-1080.922	3	-.063	3	-.288	3	0	5	0	3	0	2
137		12	max	597.673	2	.128	2	.014	9	0	3	0	4	0	12
138			min	-1080.821	3	-.096	3	-.297	5	0	5	0	3	0	2
139		13	max	597.808	2	.083	2	.014	9	0	3	0	4	0	12
140			min	-1080.719	3	-.13	3	-.42	5	0	5	0	3	0	2
141		14	max	597.943	2	.038	2	.014	9	0	3	0	4	0	12
142			min	-1080.618	3	-.164	3	-.543	5	0	5	0	3	0	2





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	598.078	2	-0.006	2	.014	9	0	3	0	4	0	12
144			min	-1080.517	3	-.197	3	-.667	5	0	5	0	3	0	2
145		16	max	598.212	2	-.051	2	.014	9	0	3	0	4	0	12
146			min	-1080.416	3	-.231	3	-.79	5	0	5	0	3	0	2
147		17	max	598.347	2	-.071	15	.014	9	0	3	0	4	0	3
148			min	-1080.315	3	-.264	3	-.913	5	0	5	0	3	0	2
149		18	max	598.482	2	-.085	15	.014	9	0	3	0	4	0	3
150			min	-1080.214	3	-.32	4	-1.036	5	0	5	0	3	0	2
151		19	max	598.617	2	-.099	15	.014	9	0	3	0	9	0	3
152			min	-1080.113	3	-.377	4	-1.159	5	0	5	0	3	0	2
153	M7	1	max	735.672	2	1.759	4	.055	3	0	9	0	4	0	2
154			min	-624.42	3	.422	15	-1.296	4	0	3	0	3	0	3
155		2	max	735.602	2	1.582	4	.055	3	0	9	0	1	0	2
156			min	-624.472	3	.381	15	-1.162	4	0	3	0	3	0	3
157		3	max	735.532	2	1.406	4	.055	3	0	9	0	1	0	2
158			min	-624.525	3	.339	15	-1.028	4	0	3	0	3	0	3
159		4	max	735.462	2	1.229	4	.055	3	0	9	0	1	0	2
160			min	-624.577	3	.298	15	-.895	4	0	3	0	3	0	3
161		5	max	735.392	2	1.053	4	.055	3	0	9	0	1	0	15
162			min	-624.63	3	.256	15	-.761	4	0	3	0	5	0	3
163		6	max	735.322	2	.877	4	.055	3	0	9	0	1	0	15
164			min	-624.682	3	.215	15	-.627	4	0	3	0	5	0	3
165		7	max	735.252	2	.7	4	.055	3	0	9	0	1	0	15
166			min	-624.735	3	.173	15	-.494	4	0	3	0	5	0	6
167		8	max	735.182	2	.524	4	.055	3	0	9	0	1	0	15
168			min	-624.787	3	.13	12	-.36	4	0	3	0	5	-.001	6
169		9	max	735.112	2	.348	4	.055	3	0	9	0	1	0	15
170			min	-624.84	3	.062	12	-.226	4	0	3	0	5	-.001	6
171		10	max	735.042	2	.201	2	.055	3	0	9	0	1	0	15
172			min	-624.892	3	-.017	3	-.093	4	0	3	-.001	5	-.001	6
173		11	max	734.972	2	.063	2	.055	3	0	9	0	1	0	15
174			min	-624.945	3	-.12	3	-.015	1	0	3	-.001	5	-.001	6
175		12	max	734.902	2	-.034	15	.175	5	0	9	0	1	0	15
176			min	-624.997	3	-.224	3	-.015	1	0	3	-.001	5	-.001	6
177		13	max	734.832	2	-.075	15	.309	5	0	9	0	1	0	15
178			min	-625.05	3	-.359	6	-.015	1	0	3	0	5	-.001	6
179		14	max	734.762	2	-.117	15	.443	5	0	9	0	1	0	15
180			min	-625.102	3	-.535	6	-.015	1	0	3	0	5	-.001	6
181		15	max	734.692	2	-.158	15	.576	5	0	9	0	1	0	15
182			min	-625.155	3	-.711	6	-.015	1	0	3	0	5	0	6
183		16	max	734.622	2	-.2	15	.71	5	0	9	0	1	0	15
184			min	-625.207	3	-.888	6	-.015	1	0	3	0	5	0	6
185		17	max	734.552	2	-.241	15	.844	5	0	9	0	9	0	15
186			min	-625.26	3	-1.064	6	-.015	1	0	3	0	5	0	6
187		18	max	734.482	2	-.283	15	.977	5	0	9	0	9	0	15
188			min	-625.312	3	-1.241	6	-.015	1	0	3	0	3	0	6
189		19	max	734.412	2	-.324	15	1.111	5	0	9	0	9	0	1
190			min	-625.365	3	-1.417	6	-.015	1	0	3	0	3	0	1
191	M8	1	max	630.74	1	0	1	.126	9	0	1	0	4	0	1
192			min	17.616	15	0	1	-14.825	4	0	1	0	3	0	1
193		2	max	630.805	1	0	1	.126	9	0	1	0	9	0	1
194			min	17.636	15	0	1	-14.881	4	0	1	-.001	4	0	1
195		3	max	630.87	1	0	1	.126	9	0	1	0	9	0	1
196			min	17.655	15	0	1	-14.937	4	0	1	-.003	4	0	1
197		4	max	630.934	1	0	1	.126	9	0	1	0	9	0	1
198			min	17.675	15	0	1	-14.993	4	0	1	-.004	4	0	1
199		5	max	630.999	1	0	1	.126	9	0	1	0	9	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	17.694	15	0	1	-15.049	4	0	1	-.005	4	0	1
201		6	max	631.064	1	0	1	.126	9	0	1	0	9	0	1
202			min	17.714	15	0	1	-15.105	4	0	1	-.007	4	0	1
203		7	max	631.128	1	0	1	.126	9	0	1	0	9	0	1
204			min	17.733	15	0	1	-15.162	4	0	1	-.008	4	0	1
205		8	max	631.193	1	0	1	.126	9	0	1	0	9	0	1
206			min	17.753	15	0	1	-15.218	4	0	1	-.009	4	0	1
207		9	max	631.258	1	0	1	.126	9	0	1	0	9	0	1
208			min	17.772	15	0	1	-15.274	4	0	1	-.011	4	0	1
209		10	max	631.322	1	0	1	.126	9	0	1	0	9	0	1
210			min	17.792	15	0	1	-15.33	4	0	1	-.012	4	0	1
211		11	max	631.387	1	0	1	.126	9	0	1	0	9	0	1
212			min	17.812	15	0	1	-15.386	4	0	1	-.013	4	0	1
213		12	max	631.452	1	0	1	.126	9	0	1	0	9	0	1
214			min	17.831	15	0	1	-15.442	4	0	1	-.015	4	0	1
215		13	max	631.517	1	0	1	.126	9	0	1	0	9	0	1
216			min	17.851	15	0	1	-15.498	4	0	1	-.016	4	0	1
217		14	max	631.581	1	0	1	.126	9	0	1	0	9	0	1
218			min	17.87	15	0	1	-15.554	4	0	1	-.018	4	0	1
219		15	max	631.646	1	0	1	.126	9	0	1	0	9	0	1
220			min	17.89	15	0	1	-15.61	4	0	1	-.019	4	0	1
221		16	max	631.711	1	0	1	.126	9	0	1	0	9	0	1
222			min	17.909	15	0	1	-15.666	4	0	1	-.02	4	0	1
223		17	max	631.775	1	0	1	.126	9	0	1	0	9	0	1
224			min	17.929	15	0	1	-15.722	4	0	1	-.022	4	0	1
225		18	max	631.84	1	0	1	.126	9	0	1	0	9	0	1
226			min	17.948	15	0	1	-15.778	4	0	1	-.023	4	0	1
227		19	max	631.905	1	0	1	.126	9	0	1	0	9	0	1
228			min	17.968	15	0	1	-15.834	4	0	1	-.025	4	0	1
229	M10	1	max	194.818	2	.712	4	1.166	5	0	1	0	1	0	1
230			min	-274.413	3	.182	15	-.099	1	-.001	5	0	3	0	1
231		2	max	194.953	2	.655	4	1.043	5	0	1	0	4	0	15
232			min	-274.312	3	.169	15	-.099	1	-.001	5	0	3	0	4
233		3	max	195.088	2	.597	4	.92	5	0	1	0	4	0	15
234			min	-274.211	3	.155	15	-.099	1	-.001	5	0	3	0	4
235		4	max	195.223	2	.54	4	.797	5	0	1	0	4	0	15
236			min	-274.11	3	.142	15	-.099	1	-.001	5	0	3	0	4
237		5	max	195.358	2	.482	4	.674	5	0	1	0	4	0	15
238			min	-274.009	3	.128	15	-.099	1	-.001	5	0	3	0	4
239		6	max	195.492	2	.425	4	.551	5	0	1	0	4	0	15
240			min	-273.908	3	.115	15	-.099	1	-.001	5	0	3	0	4
241		7	max	195.627	2	.368	4	.427	5	0	1	0	4	0	15
242			min	-273.807	3	.101	15	-.099	1	-.001	5	0	3	0	4
243		8	max	195.762	2	.31	4	.304	5	0	1	0	4	0	15
244			min	-273.705	3	.088	12	-.099	1	-.001	5	0	3	0	4
245		9	max	195.897	2	.253	4	.181	5	0	1	0	4	0	15
246			min	-273.604	3	.065	12	-.099	1	-.001	5	0	3	0	4
247		10	max	196.032	2	.195	4	.058	5	0	1	0	5	0	15
248			min	-273.503	3	.043	12	-.099	1	-.001	5	0	3	0	4
249		11	max	196.167	2	.138	4	.004	3	0	1	0	5	0	15
250			min	-273.402	3	.02	12	-.099	1	-.001	5	0	3	0	4
251		12	max	196.302	2	.08	4	.004	3	0	1	0	5	0	15
252			min	-273.301	3	-.005	3	-.203	4	-.001	5	0	3	0	4
253		13	max	196.437	2	.03	5	.004	3	0	1	0	5	0	15
254			min	-273.2	3	-.038	3	-.326	4	-.001	5	0	3	0	4
255		14	max	196.571	2	.009	5	.004	3	0	1	0	5	0	15
256			min	-273.099	3	-.072	3	-.449	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	196.706	2	-0.007	15	.004	3	0	1	0	5	0	12
258		min	-272.997	3	-1.106	3	-.572	4	-.001	5	0	3	0	4
259	16	max	196.841	2	-.02	15	.004	3	0	1	0	5	0	12
260		min	-272.896	3	-.151	6	-.696	4	-.001	5	0	3	0	4
261	17	max	196.976	2	-.034	15	.004	3	0	1	0	5	0	12
262		min	-272.795	3	-.209	6	-.819	4	-.001	5	0	3	0	4
263	18	max	197.111	2	-.047	15	.004	3	0	1	0	5	0	12
264		min	-272.694	3	-.266	6	-.942	4	-.001	5	0	3	0	4
265	19	max	197.246	2	-.061	15	.004	3	0	1	0	5	0	12
266		min	-272.593	3	-.324	6	-1.065	4	-.001	5	0	3	0	4
267	M11	1	max	238.472	2	1.72	.116	1	0	4	0	5	0	2
268		min	-225.442	3	.397	15	-1.264	5	0	10	0	1	0	15
269	2	max	238.402	2	1.543	6	.116	1	0	4	0	3	0	2
270		min	-225.494	3	.355	15	-1.131	5	0	10	0	1	0	3
271	3	max	238.332	2	1.367	6	.116	1	0	4	0	3	0	2
272		min	-225.547	3	.314	15	-.997	5	0	10	0	1	0	3
273	4	max	238.262	2	1.191	6	.116	1	0	4	0	3	0	15
274		min	-225.599	3	.272	15	-.863	5	0	10	0	1	0	4
275	5	max	238.192	2	1.014	6	.116	1	0	4	0	3	0	15
276		min	-225.652	3	.231	15	-.73	5	0	10	0	4	0	4
277	6	max	238.122	2	.838	6	.116	1	0	4	0	3	0	15
278		min	-225.704	3	.189	15	-.596	5	0	10	0	4	0	4
279	7	max	238.052	2	.661	6	.116	1	0	4	0	3	0	15
280		min	-225.757	3	.148	15	-.462	5	0	10	0	4	-.001	4
281	8	max	237.982	2	.485	6	.116	1	0	4	0	3	0	15
282		min	-225.809	3	.107	15	-.329	5	0	10	0	4	-.001	4
283	9	max	237.912	2	.309	6	.116	1	0	4	0	3	0	15
284		min	-225.862	3	.065	15	-.195	5	0	10	0	4	-.001	4
285	10	max	237.842	2	.144	2	.116	1	0	4	0	3	0	15
286		min	-225.914	3	.024	15	-.072	3	0	10	0	4	-.001	4
287	11	max	237.772	2	.006	2	.116	1	0	4	0	3	0	15
288		min	-225.967	3	-.055	3	-.072	3	0	10	0	4	-.001	4
289	12	max	237.702	2	-.059	15	.237	4	0	4	0	3	0	15
290		min	-226.019	3	-.221	4	-.072	3	0	10	0	4	-.001	4
291	13	max	237.632	2	-.101	15	.371	4	0	4	0	3	0	15
292		min	-226.072	3	-.397	4	-.072	3	0	10	0	4	-.001	4
293	14	max	237.562	2	-.142	15	.504	4	0	4	0	3	0	15
294		min	-226.124	3	-.574	4	-.072	3	0	10	0	4	-.001	4
295	15	max	237.492	2	-.184	15	.638	4	0	4	0	3	0	15
296		min	-226.177	3	-.75	4	-.072	3	0	10	0	4	0	4
297	16	max	237.422	2	-.225	15	.772	4	0	4	0	3	0	15
298		min	-226.229	3	-.927	4	-.072	3	0	10	0	5	0	4
299	17	max	237.352	2	-.267	15	.905	4	0	4	0	3	0	15
300		min	-226.282	3	-1.103	4	-.072	3	0	10	0	5	0	4
301	18	max	237.282	2	-.308	15	1.039	4	0	4	0	3	0	15
302		min	-226.334	3	-1.279	4	-.072	3	0	10	0	10	0	4
303	19	max	237.212	2	-.35	15	1.173	4	0	4	0	4	0	1
304		min	-226.387	3	-1.456	4	-.072	3	0	10	0	10	0	1
305	M12	1	max	231.286	1	0	.593	1	0	1	0	4	0	1
306		min	-.029	15	0	1	-13.649	5	0	1	0	3	0	1
307	2	max	231.351	1	0	1	.593	1	0	1	0	1	0	1
308		min	-.01	15	0	1	-13.705	5	0	1	-.001	5	0	1
309	3	max	231.416	1	0	1	.593	1	0	1	0	1	0	1
310		min	.01	15	0	1	-13.761	5	0	1	-.002	5	0	1
311	4	max	231.481	1	0	1	.593	1	0	1	0	1	0	1
312		min	.029	15	0	1	-13.817	5	0	1	-.004	5	0	1
313	5	max	231.545	1	0	1	.593	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314			min	.049	15	0	1	-13.873	5	0	1	-.005	5	0	1
315		6	max	231.61	1	0	1	.593	1	0	1	0	1	0	1
316			min	.068	15	0	1	-13.93	5	0	1	-.006	5	0	1
317		7	max	231.675	1	0	1	.593	1	0	1	0	1	0	1
318			min	.088	15	0	1	-13.986	5	0	1	-.007	5	0	1
319		8	max	231.739	1	0	1	.593	1	0	1	0	1	0	1
320			min	.107	15	0	1	-14.042	5	0	1	-.009	5	0	1
321		9	max	231.804	1	0	1	.593	1	0	1	0	1	0	1
322			min	.127	15	0	1	-14.098	5	0	1	-.01	5	0	1
323		10	max	231.869	1	0	1	.593	1	0	1	0	1	0	1
324			min	.146	15	0	1	-14.154	5	0	1	-.011	5	0	1
325		11	max	231.933	1	0	1	.593	1	0	1	0	1	0	1
326			min	.166	15	0	1	-14.21	5	0	1	-.012	5	0	1
327		12	max	231.998	1	0	1	.593	1	0	1	0	1	0	1
328			min	.185	15	0	1	-14.266	5	0	1	-.014	5	0	1
329		13	max	232.063	1	0	1	.593	1	0	1	0	1	0	1
330			min	.205	15	0	1	-14.322	5	0	1	-.015	5	0	1
331		14	max	232.128	1	0	1	.593	1	0	1	0	1	0	1
332			min	.224	15	0	1	-14.378	5	0	1	-.016	5	0	1
333		15	max	232.192	1	0	1	.593	1	0	1	0	1	0	1
334			min	.244	15	0	1	-14.434	5	0	1	-.018	5	0	1
335		16	max	232.257	1	0	1	.593	1	0	1	0	1	0	1
336			min	.263	15	0	1	-14.49	5	0	1	-.019	5	0	1
337		17	max	232.322	1	0	1	.593	1	0	1	0	1	0	1
338			min	.283	15	0	1	-14.546	5	0	1	-.02	5	0	1
339		18	max	232.386	1	0	1	.593	1	0	1	0	1	0	1
340			min	.303	15	0	1	-14.603	5	0	1	-.021	5	0	1
341		19	max	232.451	1	0	1	.593	1	0	1	0	1	0	1
342			min	.322	15	0	1	-14.659	5	0	1	-.023	5	0	1
343	M1	1	max	71.968	1	346.362	3	.963	10	0	2	.03	1	0	2
344			min	5.649	10	-216.035	2	-16.583	4	0	3	-.002	10	0	3
345		2	max	72.128	1	346.19	3	.963	10	0	2	.027	1	.047	2
346			min	5.782	10	-216.264	2	-16.341	4	0	3	-.002	10	-.075	3
347		3	max	120.114	3	4.526	4	.96	10	0	10	.023	1	.093	2
348			min	-29.385	2	-30.754	2	-15.3	1	0	1	-.001	10	-.149	3
349		4	max	120.234	3	4.232	4	.96	10	0	10	.02	1	.1	2
350			min	-29.225	2	-30.983	2	-15.3	1	0	1	-.001	10	-.147	3
351		5	max	120.354	3	3.939	4	.96	10	0	10	.017	1	.107	2
352			min	-29.065	2	-31.212	2	-15.3	1	0	1	-.001	10	-.146	3
353		6	max	120.474	3	3.685	14	.96	10	0	10	.013	1	.114	2
354			min	-28.905	2	-31.441	2	-15.3	1	0	1	0	10	-.144	3
355		7	max	120.594	3	3.461	14	.96	10	0	10	.01	1	.12	2
356			min	-28.745	2	-31.669	2	-15.3	1	0	1	0	10	-.142	3
357		8	max	120.714	3	3.236	14	.96	10	0	10	.007	1	.127	2
358			min	-28.585	2	-31.898	2	-15.3	1	0	1	0	10	-.14	3
359		9	max	120.834	3	3.011	14	.96	10	0	10	.004	3	.134	2
360			min	-28.424	2	-32.127	2	-15.3	1	0	1	0	10	-.138	3
361		10	max	120.954	3	2.787	14	.96	10	0	10	.002	3	.141	2
362			min	-28.264	2	-32.356	2	-15.3	1	0	1	0	10	-.136	3
363		11	max	121.075	3	2.562	14	.96	10	0	10	0	3	.148	2
364			min	-28.104	2	-32.584	2	-15.3	1	0	1	-.003	1	-.134	3
365		12	max	121.195	3	2.337	14	.96	10	0	10	0	10	.155	2
366			min	-27.944	2	-32.813	2	-15.3	1	0	1	-.007	1	-.132	3
367		13	max	121.315	3	2.112	14	.96	10	0	10	0	10	.163	2
368			min	-27.784	2	-33.042	2	-15.3	1	0	1	-.01	1	-.13	3
369		14	max	121.435	3	1.888	14	.96	10	0	10	0	10	.17	2
370			min	-27.624	2	-33.271	2	-15.3	1	0	1	-.013	1	-.128	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	121.555	3	1.663	14	.96	10	0	10	.001	10	.177	2
372		min	-27.463	2	-33.499	2	-15.3	1	0	1	-.017	1	-.125	3
373	16	max	85.217	2	175.897	2	.967	10	0	1	.001	10	.182	2
374		min	2.519	15	-209.8	3	-15.398	1	0	5	-.02	1	-.121	3
375	17	max	85.377	2	175.669	2	.967	10	0	1	.001	10	.144	2
376		min	2.567	15	-209.971	3	-15.398	1	0	5	-.023	1	-.076	3
377	18	max	-5.525	12	333.234	2	1.01	10	0	5	.002	10	.073	2
378		min	-72.141	1	-173.886	3	-24.631	4	0	2	-.027	1	-.038	3
379	19	max	-5.445	12	333.005	2	1.01	10	0	5	.002	10	0	2
380		min	-71.98	1	-174.057	3	-24.389	4	0	2	-.03	1	0	3
381	M5	1	max	181.85	1	1092.752	3	0	11	0	.028	4	0	3
382		min	-5.284	3	-670.159	2	-91.001	3	0	3	0	11	0	2
383	2	max	182.01	1	1092.58	3	0	11	0	9	.024	4	.145	2
384		min	-5.164	3	-670.388	2	-91.001	3	0	3	-.006	3	-.236	3
385	3	max	331.973	3	4.365	9	9.804	3	0	3	.02	4	.288	2
386		min	-84.425	2	-99.529	2	-15.55	4	0	4	-.025	3	-.468	3
387	4	max	332.093	3	4.175	9	9.804	3	0	3	.016	4	.31	2
388		min	-84.264	2	-99.757	2	-15.308	4	0	4	-.023	3	-.46	3
389	5	max	332.213	3	3.984	9	9.804	3	0	3	.013	4	.331	2
390		min	-84.104	2	-99.986	2	-15.066	4	0	4	-.021	3	-.452	3
391	6	max	332.333	3	3.794	9	9.804	3	0	3	.01	4	.353	2
392		min	-83.944	2	-100.215	2	-14.824	4	0	4	-.019	3	-.444	3
393	7	max	332.453	3	3.603	9	9.804	3	0	3	.007	4	.375	2
394		min	-83.784	2	-100.443	2	-14.582	4	0	4	-.017	3	-.436	3
395	8	max	332.573	3	3.412	9	9.804	3	0	3	.004	4	.396	2
396		min	-83.624	2	-100.672	2	-14.34	4	0	4	-.014	3	-.428	3
397	9	max	332.693	3	3.222	9	9.804	3	0	3	0	4	.418	2
398		min	-83.464	2	-100.901	2	-14.098	4	0	4	-.012	3	-.42	3
399	10	max	332.814	3	3.031	9	9.804	3	0	3	0	1	.44	2
400		min	-83.303	2	-101.13	2	-13.856	4	0	4	-.01	3	-.412	3
401	11	max	332.934	3	2.84	9	9.804	3	0	3	0	2	.462	2
402		min	-83.143	2	-101.358	2	-13.614	4	0	4	-.008	3	-.404	3
403	12	max	333.054	3	2.65	9	9.804	3	0	3	0	2	.484	2
404		min	-82.983	2	-101.587	2	-13.372	4	0	4	-.008	4	-.396	3
405	13	max	333.174	3	2.459	9	9.804	3	0	3	0	2	.506	2
406		min	-82.823	2	-101.816	2	-13.13	4	0	4	-.011	4	-.387	3
407	14	max	333.294	3	2.269	9	9.804	3	0	3	0	2	.528	2
408		min	-82.663	2	-102.045	2	-12.888	4	0	4	-.014	4	-.379	3
409	15	max	333.414	3	2.078	9	9.804	3	0	3	0	3	.551	2
410		min	-82.503	2	-102.273	2	-12.646	4	0	4	-.017	4	-.371	3
411	16	max	263.039	2	548.582	2	9.786	3	0	3	.002	3	.567	2
412		min	-.081	15	-591.679	3	-11.3	4	0	4	-.02	4	-.358	3
413	17	max	263.199	2	548.353	2	9.786	3	0	3	.004	3	.448	2
414		min	-.033	15	-591.851	3	-11.057	4	0	4	-.022	4	-.229	3
415	18	max	-2.41	12	1039.332	2	8.961	3	0	4	.006	3	.225	2
416		min	-181.994	1	-529.003	3	-25.373	5	0	9	-.028	4	-.114	3
417	19	max	-2.33	12	1039.103	2	8.961	3	0	4	.008	3	0	3
418		min	-181.834	1	-529.175	3	-25.131	5	0	9	-.033	4	0	2
419	M9	1	max	71.962	1	346.242	3	108.529	4	0	.002	10	0	2
420		min	1.441	15	-216.035	2	-.963	10	0	2	-.03	1	0	3
421	2	max	72.122	1	346.07	3	108.771	4	0	3	.021	5	.047	2
422		min	1.489	15	-216.264	2	-.963	10	0	2	-.027	1	-.075	3
423	3	max	119.414	3	3.928	9	15.264	1	0	1	.043	5	.093	2
424		min	-28.937	2	-30.726	2	-19.763	5	0	5	-.023	1	-.149	3
425	4	max	119.534	3	3.737	9	15.264	1	0	1	.038	5	.1	2
426		min	-28.777	2	-30.955	2	-19.521	5	0	5	-.02	1	-.147	3
427	5	max	119.655	3	3.547	9	15.264	1	0	1	.034	5	.107	2







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	32.934	4	28.755	2	10.622	4	0	3	0	5	.247	3
486		min	-.963	10	-50.422	3	-4.869	2	0	2	-.029	1	-.156	2
487	16	max	28.841	4	48.762	3	17.667	1	0	3	.005	5	.247	3
488		min	-.963	10	-32.422	2	-1.121	10	0	2	-.026	1	-.155	2
489	17	max	24.749	4	147.946	3	35.766	1	0	3	.01	5	.206	3
490		min	-.963	10	-93.6	2	1.135	10	0	2	-.015	1	-.129	2
491	18	max	20.657	4	247.13	3	53.865	1	0	3	.017	4	.124	3
492		min	-.963	10	-154.777	2	3.392	10	0	2	-.004	2	-.077	2
493	19	max	16.564	4	346.314	3	71.964	1	0	3	.03	1	0	2
494		min	-.963	10	-215.954	2	5.649	10	0	2	-.002	10	0	3
495	M16	1	max	28.387	5	333.112	2	6.599	5	0	.03	1	0	2
496		min	-15.944	1	-174.079	3	-71.97	1	0	2	-.024	5	0	3
497	2	max	24.294	5	238.307	2	7.863	5	0	3	.005	9	.062	3
498		min	-15.944	1	-125.336	3	-53.871	1	0	2	-.02	5	-.119	2
499	3	max	20.202	5	143.503	2	9.127	5	0	3	0	3	.104	3
500		min	-15.944	1	-76.594	3	-35.772	1	0	2	-.02	4	-.199	2
501	4	max	16.109	5	48.698	2	10.39	5	0	3	-.002	12	.126	3
502		min	-15.944	1	-27.851	3	-17.673	1	0	2	-.026	1	-.239	2
503	5	max	12.017	5	20.892	3	11.654	5	0	3	-.003	12	.128	3
504		min	-15.944	1	-46.107	2	-5.308	3	0	2	-.029	1	-.239	2
505	6	max	7.925	5	69.634	3	18.524	1	0	3	-.002	10	.109	3
506		min	-15.944	1	-140.911	2	-4.119	3	0	2	-.025	1	-.2	2
507	7	max	3.832	5	118.377	3	36.623	1	0	3	.002	5	.07	3
508		min	-15.944	1	-235.716	2	-2.93	3	0	2	-.014	1	-.122	2
509	8	max	2.488	3	167.12	3	54.722	1	0	3	.009	4	.01	3
510		min	-15.944	1	-330.521	2	-1.741	3	0	2	-.009	3	-.004	2
511	9	max	2.488	3	215.862	3	72.821	1	0	3	.032	1	.154	2
512		min	-15.944	1	-425.325	2	-.552	3	0	2	-.01	3	-.07	3
513	10	max	16.695	5	-6.382	15	90.92	1	0	14	.066	1	.351	2
514		min	-15.944	1	-520.13	2	-2.325	3	0	2	-.01	3	-.17	3
515	11	max	12.602	5	425.325	2	3.831	5	0	2	.032	1	.154	2
516		min	-15.933	1	-215.862	3	-72.806	1	0	3	-.009	5	-.07	3
517	12	max	8.51	5	330.521	2	5.095	5	0	2	.008	2	.01	3
518		min	-15.933	1	-167.12	3	-54.707	1	0	3	-.007	5	-.004	2
519	13	max	4.417	5	235.716	2	6.359	5	0	2	0	10	.07	3
520		min	-15.933	1	-118.377	3	-36.608	1	0	3	-.014	1	-.122	2
521	14	max	1.009	10	140.911	2	7.623	5	0	2	-.001	12	.109	3
522		min	-15.933	1	-69.634	3	-18.51	1	0	3	-.025	1	-.2	2
523	15	max	1.009	10	46.107	2	9.525	4	0	2	.002	5	.128	3
524		min	-15.933	1	-20.892	3	-4.829	2	0	3	-.029	1	-.239	2
525	16	max	1.009	10	27.851	3	17.688	1	0	2	.006	5	.126	3
526		min	-15.933	1	-48.698	2	-1.096	10	0	3	-.026	1	-.239	2
527	17	max	1.009	10	76.594	3	35.787	1	0	2	.01	5	.104	3
528		min	-16.225	4	-143.503	2	1.161	10	0	3	-.015	1	-.199	2
529	18	max	1.009	10	125.337	3	53.886	1	0	2	.017	4	.062	3
530		min	-20.318	4	-238.307	2	3.418	10	0	3	-.004	2	-.119	2
531	19	max	1.009	10	174.079	3	71.985	1	0	2	.03	1	0	2
532		min	-24.41	4	-333.112	2	5.443	12	0	3	-.002	10	0	5
533	M15	1	max	0	.792	3	.149	3	0	1	0	1	0	1
534		min	-138.476	3	0	1	0	1	0	3	0	3	0	1
535	2	max	0	1	.704	3	.149	3	0	1	0	1	0	1
536		min	-138.551	3	0	1	0	1	0	3	0	3	0	3
537	3	max	0	1	.616	3	.149	3	0	1	0	1	0	1
538		min	-138.627	3	0	1	0	1	0	3	0	3	0	3
539	4	max	0	1	.528	3	.149	3	0	1	0	1	0	1
540		min	-138.702	3	0	1	0	1	0	3	0	3	0	3
541	5	max	0	1	.44	3	.149	3	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-138.778	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.352	3	.149	3	0	1	0	1	0	1
544		min	-138.853	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.264	3	.149	3	0	1	0	3	0	1
546		min	-138.929	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.176	3	.149	3	0	1	0	3	0	1
548		min	-139.004	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.088	3	.149	3	0	1	0	3	0	1
550		min	-139.08	3	0	1	0	1	0	3	0	1	-.001	3
551	10	max	0	1	0	1	.149	3	0	1	0	3	0	1
552		min	-139.155	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.149	3	0	1	0	3	0	1
554		min	-139.231	3	-.088	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.149	3	0	1	0	3	0	1
556		min	-139.306	3	-.176	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.149	3	0	1	0	3	0	1
558		min	-139.382	3	-.264	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.149	3	0	1	0	3	0	1
560		min	-139.457	3	-.352	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.149	3	0	1	0	3	0	1
562		min	-139.533	3	-.44	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.149	3	0	1	0	3	0	1
564		min	-139.609	3	-.528	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.149	3	0	1	0	3	0	1
566		min	-139.684	3	-.616	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.149	3	0	1	0	3	0	1
568		min	-139.76	3	-.704	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.149	3	0	1	0	3	0	1
570		min	-139.835	3	-.792	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.109	.37	4	0	3	0	3	0	1
572		min	-175.118	4	0	2	-.062	3	0	1	0	4	0	1
573	2	max	0	2	1.874	4	.332	4	0	3	0	3	0	2
574		min	-175.086	4	0	2	-.062	3	0	1	0	4	0	4
575	3	max	0	2	1.64	4	.294	4	0	3	0	3	0	2
576		min	-175.054	4	0	2	-.062	3	0	1	0	4	-.001	4
577	4	max	0	2	1.406	4	.255	4	0	3	0	3	0	2
578		min	-175.022	4	0	2	-.062	3	0	1	0	1	-.002	4
579	5	max	0	2	1.171	4	.217	4	0	3	0	3	0	2
580		min	-174.991	4	0	2	-.062	3	0	1	0	1	-.002	4
581	6	max	0	2	.937	4	.179	4	0	3	0	3	0	2
582		min	-174.959	4	0	2	-.062	3	0	1	0	1	-.002	4
583	7	max	0	2	.703	4	.141	4	0	3	0	3	0	2
584		min	-174.927	4	0	2	-.062	3	0	1	0	1	-.002	4
585	8	max	0	2	.469	4	.103	4	0	3	0	5	0	2
586		min	-174.895	4	0	2	-.062	3	0	1	0	1	-.003	4
587	9	max	0	2	.234	4	.064	4	0	3	0	5	0	2
588		min	-174.864	4	0	2	-.062	3	0	1	0	1	-.003	4
589	10	max	0	2	0	1	.037	1	0	3	0	5	0	2
590		min	-174.832	4	0	1	-.062	3	0	1	0	1	-.003	4
591	11	max	.058	1	0	2	.037	1	0	3	0	5	0	2
592		min	-174.8	4	-.234	4	-.062	3	0	1	0	1	-.003	4
593	12	max	.159	1	0	2	.037	1	0	3	0	5	0	2
594		min	-174.768	4	-.469	4	-.062	3	0	1	0	1	-.003	4
595	13	max	.26	1	0	2	.037	1	0	3	0	5	0	2
596		min	-174.737	4	-.703	4	-.09	5	0	1	0	3	-.002	4
597	14	max	.36	1	0	2	.037	1	0	3	0	5	0	2
598		min	-174.705	4	-.937	4	-.128	5	0	1	0	3	-.002	4





Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.461	1	0	2	.037	1	0	3	0	5	0	2
600		min	-174.673	4	-1.171	4	-.167	5	0	1	0	3	-.002	4
601	16	max	.562	1	0	2	.037	1	0	3	0	1	0	2
602		min	-174.657	5	-1.406	4	-.205	5	0	1	0	3	-.002	4
603	17	max	.662	1	0	2	.037	1	0	3	0	1	0	2
604		min	-174.707	5	-1.64	4	-.243	5	0	1	0	3	-.001	4
605	18	max	.763	1	0	2	.037	1	0	3	0	1	0	2
606		min	-174.758	5	-1.874	4	-.281	5	0	1	0	4	0	4
607	19	max	.864	1	0	2	.037	1	0	3	0	1	0	1
608		min	-174.808	5	-2.109	4	-.319	5	0	1	0	4	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.011	2	.002	9	8.319e-4	5	NC	3	NC	1	
2			min	-.004	3	-.011	3	-.01	5	-2.533e-4	1	3888.002	2	NC	1	
3			2	max	.002	2	.01	2	.002	9	8.533e-4	5	NC	3	NC	1
4				min	-.004	3	-.011	3	-.01	5	-2.413e-4	1	4251.022	2	NC	1
5			3	max	.002	2	.009	2	.002	9	8.747e-4	5	NC	3	NC	1
6				min	-.003	3	-.01	3	-.01	5	-2.292e-4	1	4684.139	2	NC	1
7			4	max	.002	2	.008	2	.002	9	8.962e-4	5	NC	1	NC	1
8				min	-.003	3	-.01	3	-.009	5	-2.172e-4	1	5204.413	2	NC	1
9			5	max	.002	2	.007	2	.002	9	9.176e-4	5	NC	1	NC	1
10				min	-.003	3	-.01	3	-.009	5	-2.051e-4	1	5834.62	2	NC	1
11			6	max	.001	2	.006	2	.001	9	9.39e-4	5	NC	1	NC	1
12				min	-.003	3	-.009	3	-.009	5	-1.931e-4	1	6605.653	2	NC	1
13			7	max	.001	2	.006	2	.001	9	9.604e-4	5	NC	1	NC	1
14				min	-.003	3	-.009	3	-.009	5	-1.81e-4	1	7560.201	2	NC	1
15			8	max	.001	2	.005	2	.001	9	9.819e-4	5	NC	1	NC	1
16				min	-.002	3	-.008	3	-.008	5	-1.69e-4	1	8758.556	2	NC	1
17			9	max	.001	2	.004	2	0	9	1.003e-3	5	NC	1	NC	1
18				min	-.002	3	-.007	3	-.008	5	-1.569e-4	1	NC	1	NC	1
19			10	max	.001	2	.003	2	0	9	1.025e-3	5	NC	1	NC	1
20				min	-.002	3	-.007	3	-.007	5	-1.449e-4	1	NC	1	NC	1
21		11	max	0	2	.003	2	0	9	1.046e-3	5	NC	1	NC	1	
22			min	-.002	3	-.006	3	-.007	5	-1.329e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	0	9	1.068e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.006	5	-1.208e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	9	1.089e-3	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.005	5	-1.088e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	9	1.11e-3	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.005	5	-9.672e-5	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	9	1.132e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.004	5	-8.467e-5	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	9	1.153e-3	5	NC	1	NC	1	
32			min	0	3	-.003	3	-.003	5	-7.263e-5	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	9	1.175e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.002	5	-6.058e-5	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	9	1.196e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-4.854e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.218e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.742e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.793e-5	9	NC	1	NC	1	
40			min	0	1	0	1	0	1	-5.82e-4	5	NC	1	NC	1	
41		2	max	0	3	0	2	.003	5	2.439e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	9	-5.853e-4	5	NC	1	NC	1	



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	3.123e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	9	-5.886e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.009	5	3.806e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	9	-5.918e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.012	5	4.489e-5	1	NC	1	NC	1
48			min	0	2	-.004	3	0	9	-5.951e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.015	4	5.172e-5	1	NC	1	NC	1
50			min	0	2	-.005	3	0	9	-5.984e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.017	4	5.855e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-6.016e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.02	4	6.539e-5	1	NC	1	NC	1
54			min	-.001	2	-.006	3	0	9	-6.049e-4	5	NC	1	NC	1
55		9	max	.001	3	.001	2	.023	4	7.222e-5	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	10	-6.081e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.025	4	7.905e-5	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-6.114e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.028	4	8.588e-5	1	NC	1	NC	1
60			min	-.002	2	-.008	3	0	10	-6.147e-4	5	NC	1	NC	1
61		12	max	.002	3	.003	2	.03	4	9.271e-5	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	10	-6.179e-4	5	NC	1	NC	1
63		13	max	.002	3	.004	2	.033	4	9.954e-5	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	10	-6.212e-4	5	NC	1	NC	1
65		14	max	.002	3	.005	2	.035	4	1.064e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	10	-6.245e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.037	4	1.132e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	10	-6.277e-4	5	8393.561	2	NC	1
69		16	max	.002	3	.006	2	.039	4	1.2e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	10	-6.31e-4	5	7137.321	2	NC	1
71		17	max	.002	3	.007	2	.041	4	1.269e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-6.342e-4	5	6160.5	2	NC	1
73		18	max	.002	3	.009	2	.043	4	1.337e-4	1	NC	1	NC	1
74			min	-.003	2	-.009	3	0	10	-6.375e-4	5	5392.752	2	NC	1
75		19	max	.003	3	.01	2	.045	4	1.405e-4	1	NC	3	NC	1
76			min	-.003	2	-.009	3	0	10	-6.408e-4	5	4784.229	2	NC	1
77	M4	1	max	.001	1	.013	2	0	10	3.527e-3	5	NC	1	NC	1
78			min	0	15	-.011	3	-.047	4	-1.782e-4	1	NC	1	409.605	4
79		2	max	.001	1	.012	2	0	10	3.527e-3	5	NC	1	NC	1
80			min	0	15	-.011	3	-.043	4	-1.782e-4	1	NC	1	446.464	4
81		3	max	0	1	.011	2	0	10	3.527e-3	5	NC	1	NC	1
82			min	0	15	-.01	3	-.039	4	-1.782e-4	1	NC	1	490.326	4
83		4	max	0	1	.011	2	0	10	3.527e-3	5	NC	1	NC	1
84			min	0	15	-.009	3	-.036	4	-1.782e-4	1	NC	1	543.036	4
85		5	max	0	1	.01	2	0	10	3.527e-3	5	NC	1	NC	1
86			min	0	15	-.009	3	-.032	4	-1.782e-4	1	NC	1	607.106	4
87		6	max	0	1	.009	2	0	10	3.527e-3	5	NC	1	NC	1
88			min	0	15	-.008	3	-.028	4	-1.782e-4	1	NC	1	686.029	4
89		7	max	0	1	.008	2	0	10	3.527e-3	5	NC	1	NC	1
90			min	0	15	-.007	3	-.025	4	-1.782e-4	1	NC	1	784.782	4
91		8	max	0	1	.008	2	0	10	3.527e-3	5	NC	1	NC	1
92			min	0	15	-.007	3	-.021	4	-1.782e-4	1	NC	1	910.648	4
93		9	max	0	1	.007	2	0	10	3.527e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.018	4	-1.782e-4	1	NC	1	1074.645	4
95		10	max	0	1	.006	2	0	10	3.527e-3	5	NC	1	NC	1
96			min	0	15	-.006	3	-.015	4	-1.782e-4	1	NC	1	1294.09	4
97		11	max	0	1	.006	2	0	10	3.527e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.012	4	-1.782e-4	1	NC	1	1597.503	4
99		12	max	0	1	.005	2	0	10	3.527e-3	5	NC	1	NC	1

***Envelope Member Section Deflections (Continued)***

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100			min	0	15	-.004	3	-.009	4	-1.782e-4	1	NC	1	2034.626	4
101		13	max	0	1	.004	2	0	10	3.527e-3	5	NC	1	NC	1
102			min	0	15	-.004	3	-.007	4	-1.782e-4	1	NC	1	2698.759	4
103		14	max	0	1	.004	2	0	10	3.527e-3	5	NC	1	NC	1
104			min	0	15	-.003	3	-.005	4	-1.782e-4	1	NC	1	3782.337	4
105		15	max	0	1	.003	2	0	10	3.527e-3	5	NC	1	NC	1
106			min	0	15	-.002	3	-.003	4	-1.782e-4	1	NC	1	5737.419	4
107		16	max	0	1	.002	2	0	10	3.527e-3	5	NC	1	NC	1
108			min	0	15	-.002	3	-.002	4	-1.782e-4	1	NC	1	9849.021	4
109		17	max	0	1	.001	2	0	10	3.527e-3	5	NC	1	NC	1
110			min	0	15	-.001	3	0	4	-1.782e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	10	3.527e-3	5	NC	1	NC	1
112			min	0	15	0	3	0	4	-1.782e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	3.527e-3	5	NC	1	NC	1
114			min	0	1	0	1	0	1	-1.782e-4	1	NC	1	NC	1
115	M6	1	max	.006	2	.034	2	0	9	8.791e-4	4	NC	3	NC	1
116			min	-.011	3	-.033	3	-.01	5	-3.473e-7	9	1253.811	2	5628.012	3
117		2	max	.006	2	.032	2	0	9	9.014e-4	4	NC	3	NC	1
118			min	-.011	3	-.031	3	-.01	5	-1.149e-6	1	1343.617	2	5945.073	3
119		3	max	.006	2	.029	2	0	9	9.236e-4	4	NC	3	NC	1
120			min	-.01	3	-.03	3	-.01	5	-2.089e-6	1	1446.782	2	6326.313	3
121		4	max	.005	2	.027	2	0	9	9.459e-4	4	NC	3	NC	1
122			min	-.009	3	-.028	3	-.01	5	-3.03e-6	1	1565.982	2	6784.81	3
123		5	max	.005	2	.025	2	0	9	9.682e-4	4	NC	3	NC	1
124			min	-.009	3	-.026	3	-.009	5	-3.971e-6	1	1704.663	2	7338.052	3
125		6	max	.005	2	.023	2	0	9	9.904e-4	4	NC	3	NC	1
126			min	-.008	3	-.024	3	-.009	5	-4.911e-6	1	1867.334	2	8009.692	3
127		7	max	.004	2	.021	2	0	9	1.013e-3	4	NC	3	NC	1
128			min	-.008	3	-.023	3	-.009	5	-5.852e-6	1	2060.014	2	8832.24	3
129		8	max	.004	2	.019	2	0	9	1.035e-3	4	NC	3	NC	1
130			min	-.007	3	-.021	3	-.009	5	-6.792e-6	1	2290.908	2	9851.293	3
131		9	max	.003	2	.017	2	0	9	1.057e-3	4	NC	3	NC	1
132			min	-.006	3	-.019	3	-.008	5	-7.733e-6	1	2571.498	2	NC	1
133		10	max	.003	2	.015	2	0	9	1.079e-3	4	NC	3	NC	1
134			min	-.006	3	-.017	3	-.008	5	-8.674e-6	1	2918.364	2	NC	1
135		11	max	.003	2	.013	2	0	9	1.102e-3	4	NC	3	NC	1
136			min	-.005	3	-.015	3	-.007	5	-9.614e-6	1	3356.355	2	NC	1
137		12	max	.002	2	.011	2	0	9	1.124e-3	4	NC	3	NC	1
138			min	-.004	3	-.013	3	-.006	5	-1.055e-5	1	3924.497	2	NC	1
139		13	max	.002	2	.009	2	0	9	1.146e-3	4	NC	3	NC	1
140			min	-.004	3	-.012	3	-.006	5	-1.15e-5	1	4687.784	2	NC	1
141		14	max	.002	2	.007	2	0	9	1.168e-3	4	NC	1	NC	1
142			min	-.003	3	-.01	3	-.005	5	-1.244e-5	1	5763.146	2	NC	1
143		15	max	.001	2	.006	2	0	9	1.191e-3	4	NC	1	NC	1
144			min	-.003	3	-.008	3	-.004	5	-1.338e-5	1	7384.331	2	NC	1
145		16	max	.001	2	.004	2	0	9	1.213e-3	4	NC	1	NC	1
146			min	-.002	3	-.006	3	-.003	5	-1.432e-5	1	NC	1	NC	1
147		17	max	0	2	.003	2	0	9	1.235e-3	4	NC	1	NC	1
148			min	-.001	3	-.004	3	-.002	5	-1.526e-5	1	NC	1	NC	1
149		18	max	0	2	.001	2	0	9	1.258e-3	4	NC	1	NC	1
150			min	0	3	-.002	3	-.001	5	-1.62e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.28e-3	4	NC	1	NC	1
152			min	0	1	0	1	0	1	-1.714e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	8.217e-6	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-6.117e-4	4	NC	1	NC	1
155		2	max	0	3	.001	2	.003	4	7.904e-6	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.057e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.006	4	7.592e-6	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-5.997e-4	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.009	4	7.279e-6	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-5.938e-4	4	NC	1	NC	1
161		5	max	.002	3	.005	2	.012	4	6.966e-6	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	9	-5.878e-4	4	8680.546	2	NC	1
163		6	max	.002	3	.007	2	.015	4	2.885e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	9	-5.818e-4	4	6944.191	2	NC	1
165		7	max	.002	3	.008	2	.018	4	5.714e-5	3	NC	1	NC	1
166			min	-.003	2	-.012	3	0	9	-5.759e-4	4	5757.721	2	NC	1
167		8	max	.003	3	.009	2	.021	4	8.544e-5	3	NC	1	NC	1
168			min	-.003	2	-.014	3	0	9	-5.699e-4	4	4888.913	2	NC	1
169		9	max	.003	3	.011	2	.024	4	1.137e-4	3	NC	3	NC	1
170			min	-.004	2	-.015	3	0	9	-5.639e-4	4	4221.993	2	NC	1
171		10	max	.004	3	.012	2	.026	4	1.42e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	0	9	-5.58e-4	4	3692.725	2	NC	1
173		11	max	.004	3	.014	2	.029	4	1.703e-4	3	NC	3	NC	1
174			min	-.005	2	-.018	3	0	9	-5.52e-4	4	3262.489	2	NC	1
175		12	max	.004	3	.016	2	.031	4	1.986e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	0	9	-5.46e-4	4	2906.538	2	NC	1
177		13	max	.005	3	.018	2	.034	4	2.269e-4	3	NC	3	NC	1
178			min	-.006	2	-.021	3	0	9	-5.401e-4	4	2608.156	2	NC	1
179		14	max	.005	3	.02	2	.036	4	2.552e-4	3	NC	3	NC	1
180			min	-.006	2	-.022	3	0	9	-5.341e-4	4	2355.55	2	NC	1
181		15	max	.006	3	.022	2	.038	4	2.835e-4	3	NC	3	NC	1
182			min	-.006	2	-.023	3	0	9	-5.281e-4	4	2140.09	2	NC	1
183		16	max	.006	3	.024	2	.04	4	3.118e-4	3	NC	3	NC	1
184			min	-.007	2	-.024	3	0	9	-5.222e-4	4	1955.271	2	NC	1
185		17	max	.006	3	.026	2	.042	4	3.401e-4	3	NC	3	NC	1
186			min	-.007	2	-.025	3	0	9	-5.162e-4	4	1796.066	2	NC	1
187		18	max	.007	3	.028	2	.044	4	3.683e-4	3	NC	3	NC	1
188			min	-.008	2	-.026	3	0	9	-5.102e-4	4	1658.521	2	NC	1
189		19	max	.007	3	.03	2	.046	4	3.966e-4	3	NC	3	NC	1
190			min	-.008	2	-.027	3	0	9	-5.043e-4	4	1539.477	2	NC	1
191	M8	1	max	.003	1	.039	2	0	9	3.387e-3	4	NC	1	NC	1
192			min	0	15	-.033	3	-.048	4	-2.877e-4	3	NC	1	402.966	4
193		2	max	.003	1	.037	2	0	9	3.387e-3	4	NC	1	NC	1
194			min	0	15	-.031	3	-.044	4	-2.877e-4	3	NC	1	439.228	4
195		3	max	.003	1	.035	2	0	9	3.387e-3	4	NC	1	NC	1
196			min	0	15	-.029	3	-.04	4	-2.877e-4	3	NC	1	482.381	4
197		4	max	.003	1	.033	2	0	9	3.387e-3	4	NC	1	NC	1
198			min	0	15	-.027	3	-.036	4	-2.877e-4	3	NC	1	534.239	4
199		5	max	.002	1	.03	2	0	9	3.387e-3	4	NC	1	NC	1
200			min	0	15	-.026	3	-.032	4	-2.877e-4	3	NC	1	597.274	4
201		6	max	.002	1	.028	2	0	9	3.387e-3	4	NC	1	NC	1
202			min	0	15	-.024	3	-.029	4	-2.877e-4	3	NC	1	674.924	4
203		7	max	.002	1	.026	2	0	9	3.387e-3	4	NC	1	NC	1
204			min	0	15	-.022	3	-.025	4	-2.877e-4	3	NC	1	772.082	4
205		8	max	.002	1	.024	2	0	9	3.387e-3	4	NC	1	NC	1
206			min	0	15	-.02	3	-.022	4	-2.877e-4	3	NC	1	895.917	4
207		9	max	.002	1	.022	2	0	9	3.387e-3	4	NC	1	NC	1
208			min	0	15	-.018	3	-.018	4	-2.877e-4	3	NC	1	1057.268	4
209		10	max	.002	1	.02	2	0	9	3.387e-3	4	NC	1	NC	1
210			min	0	15	-.016	3	-.015	4	-2.877e-4	3	NC	1	1273.174	4
211		11	max	.001	1	.017	2	0	9	3.387e-3	4	NC	1	NC	1
212			min	0	15	-.015	3	-.012	4	-2.877e-4	3	NC	1	1571.694	4
213		12	max	.001	1	.015	2	0	9	3.387e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.424e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.776e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.008	4	2.724e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-6.147e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.01	4	2.712e-6	10	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-6.518e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.013	4	3.108e-6	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-6.889e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.015	5	3.504e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-7.261e-4	4	NC	1	NC	1
281		8	max	0	3	.001	2	.018	5	3.9e-6	10	NC	1	NC	1
282			min	-.001	2	-.006	3	-.003	3	-7.632e-4	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.02	5	4.296e-6	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	3	-8.003e-4	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.022	5	4.691e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-8.375e-4	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.025	5	5.087e-6	10	NC	1	NC	1
288			min	-.002	2	-.008	3	-.003	3	-8.746e-4	4	NC	1	NC	1
289		12	max	.002	3	.003	2	.027	5	5.483e-6	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	3	-9.117e-4	4	NC	1	NC	1
291		13	max	.002	3	.004	2	.029	5	5.879e-6	10	NC	1	NC	1
292			min	-.002	2	-.009	3	-.003	3	-9.488e-4	4	NC	1	NC	1
293		14	max	.002	3	.005	2	.031	5	6.275e-6	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.003	3	-9.86e-4	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.033	5	6.671e-6	10	NC	1	NC	1
296			min	-.002	2	-.009	3	-.003	3	-1.023e-3	4	8404.567	2	NC	1
297		16	max	.002	3	.006	2	.035	5	7.067e-6	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	3	-1.06e-3	4	7145.839	2	NC	1
299		17	max	.002	3	.007	2	.037	5	7.463e-6	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.003	3	-1.097e-3	4	6167.264	2	NC	1
301		18	max	.002	3	.009	2	.039	5	7.859e-6	10	NC	1	NC	1
302			min	-.003	2	-.009	3	-.003	3	-1.134e-3	4	5398.255	2	NC	1
303		19	max	.003	3	.01	2	.041	5	8.255e-6	10	NC	3	NC	1
304			min	-.003	2	-.009	3	-.003	3	-1.172e-3	4	4788.812	2	NC	1
305	M12	1	max	.001	1	.013	2	.002	1	4.008e-3	4	NC	1	NC	1
306			min	0	15	-.011	3	-.044	5	-1.093e-5	10	NC	1	436.842	5
307		2	max	.001	1	.012	2	.002	1	4.008e-3	4	NC	1	NC	1
308			min	0	15	-.011	3	-.041	5	-1.093e-5	10	NC	1	476.14	5
309		3	max	0	1	.011	2	.002	1	4.008e-3	4	NC	1	NC	1
310			min	0	15	-.01	3	-.037	5	-1.093e-5	10	NC	1	522.904	5
311		4	max	0	1	.01	2	.001	1	4.008e-3	4	NC	1	NC	1
312			min	0	15	-.009	3	-.033	5	-1.093e-5	10	NC	1	579.1	5
313		5	max	0	1	.01	2	.001	1	4.008e-3	4	NC	1	NC	1
314			min	0	15	-.009	3	-.03	5	-1.093e-5	10	NC	1	647.405	5
315		6	max	0	1	.009	2	.001	1	4.008e-3	4	NC	1	NC	1
316			min	0	15	-.008	3	-.026	5	-1.093e-5	10	NC	1	731.545	5
317		7	max	0	1	.008	2	0	1	4.008e-3	4	NC	1	NC	1
318			min	0	15	-.008	3	-.023	5	-1.093e-5	10	NC	1	836.822	5
319		8	max	0	1	.008	2	0	1	4.008e-3	4	NC	1	NC	1
320			min	0	15	-.007	3	-.02	5	-1.093e-5	10	NC	1	971.002	5
321		9	max	0	1	.007	2	0	1	4.008e-3	4	NC	1	NC	1
322			min	0	15	-.006	3	-.017	5	-1.093e-5	10	NC	1	1145.827	5
323		10	max	0	1	.006	2	0	1	4.008e-3	4	NC	1	NC	1
324			min	0	15	-.006	3	-.014	5	-1.093e-5	10	NC	1	1379.757	5
325		11	max	0	1	.006	2	0	1	4.008e-3	4	NC	1	NC	1
326			min	0	15	-.005	3	-.011	5	-1.093e-5	10	NC	1	1703.192	5
327		12	max	0	1	.005	2	0	1	4.008e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	15	-.004	3	-.009	5	-1.093e-5	10	NC	1	2169.151	5
329		13	max	0	1	.004	2	0	1	4.008e-3	4	NC	1	NC	1
330			min	0	15	-.004	3	-.007	5	-1.093e-5	10	NC	1	2877.081	5
331		14	max	0	1	.003	2	0	1	4.008e-3	4	NC	1	NC	1
332			min	0	15	-.003	3	-.005	5	-1.093e-5	10	NC	1	4032.093	5
333		15	max	0	1	.003	2	0	1	4.008e-3	4	NC	1	NC	1
334			min	0	15	-.003	3	-.003	5	-1.093e-5	10	NC	1	6116.019	5
335		16	max	0	1	.002	2	0	1	4.008e-3	4	NC	1	NC	1
336			min	0	15	-.002	3	-.002	5	-1.093e-5	10	NC	1	NC	1
337		17	max	0	1	.001	2	0	1	4.008e-3	4	NC	1	NC	1
338			min	0	15	-.001	3	0	5	-1.093e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	4.008e-3	4	NC	1	NC	1
340			min	0	15	0	3	0	5	-1.093e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	4.008e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.093e-5	10	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.006	5	5.381e-3	2	NC	1	NC	1
344			min	-.01	2	-.022	2	0	9	-8.079e-3	3	NC	1	NC	1
345		2	max	.01	3	.017	3	.008	5	2.657e-3	2	NC	4	NC	1
346			min	-.01	2	-.013	2	-.002	9	-3.983e-3	3	5325.951	2	NC	1
347		3	max	.01	3	.007	3	.01	5	2.945e-4	5	NC	4	NC	1
348			min	-.01	2	-.005	2	-.002	9	-1.207e-4	1	2730.822	2	NC	1
349		4	max	.01	3	.002	2	.012	5	2.987e-4	5	NC	4	NC	1
350			min	-.01	2	-.002	3	-.002	9	-1.011e-4	1	1858.007	3	8215.359	5
351		5	max	.01	3	.009	2	.014	5	3.03e-4	5	NC	4	NC	1
352			min	-.01	2	-.009	3	-.003	9	-8.206e-5	9	1452.373	3	5812.863	5
353		6	max	.01	3	.015	2	.017	5	3.073e-4	5	NC	4	NC	1
354			min	-.01	2	-.014	3	-.002	9	-6.624e-5	9	1234.154	3	4426.529	5
355		7	max	.01	3	.019	2	.02	5	3.115e-4	5	NC	4	NC	1
356			min	-.01	2	-.019	3	-.002	9	-5.041e-5	9	1106.389	3	3538.262	5
357		8	max	.01	3	.022	2	.022	5	3.158e-4	5	NC	4	NC	1
358			min	-.01	2	-.022	3	-.002	9	-3.459e-5	9	1031.345	3	2928.571	5
359		9	max	.01	3	.025	2	.025	5	3.2e-4	5	NC	4	NC	1
360			min	-.01	2	-.024	3	-.001	9	-1.877e-5	9	992.115	3	2489.212	5
361		10	max	.01	3	.026	2	.028	5	3.259e-4	4	NC	4	NC	1
362			min	-.01	2	-.024	3	0	9	-2.941e-6	9	981.118	3	2145.273	4
363		11	max	.01	3	.025	2	.031	4	3.358e-4	4	NC	4	NC	1
364			min	-.01	2	-.023	3	0	9	-1.644e-6	10	990.865	2	1881.016	4
365		12	max	.009	3	.023	2	.034	4	3.457e-4	4	NC	4	NC	1
366			min	-.01	2	-.021	3	0	10	-2.933e-6	10	1027.701	2	1677.075	4
367		13	max	.009	3	.02	2	.037	4	3.555e-4	4	NC	4	NC	1
368			min	-.01	2	-.018	3	0	10	-4.223e-6	10	1103.273	2	1517.07	4
369		14	max	.009	3	.016	2	.04	4	3.654e-4	4	NC	4	NC	1
370			min	-.01	2	-.014	3	0	10	-5.512e-6	10	1237.83	2	1390.065	4
371		15	max	.009	3	.01	2	.042	4	3.753e-4	4	NC	4	NC	1
372			min	-.01	2	-.008	3	0	10	-6.801e-6	10	1461.834	3	1288.551	4
373		16	max	.009	3	.002	2	.045	4	5.592e-4	4	NC	4	NC	1
374			min	-.01	2	-.002	3	0	10	-7.745e-6	10	1846.66	3	1207.25	4
375		17	max	.009	3	.006	3	.047	4	4.886e-3	4	NC	4	NC	1
376			min	-.01	2	-.008	2	0	10	-1.995e-5	9	2679.617	3	1142.483	4
377		18	max	.009	3	.015	3	.049	4	4.005e-3	2	NC	1	NC	1
378			min	-.01	2	-.019	2	0	10	-2.245e-3	3	5254.415	3	1091.353	4
379		19	max	.009	3	.024	3	.051	4	8.081e-3	2	NC	1	NC	1
380			min	-.01	2	-.03	2	0	9	-4.624e-3	3	5629.054	2	1053.133	4
381	M5	1	max	.028	3	.083	3	.006	5	2.129e-5	4	NC	1	NC	1
382			min	-.031	2	-.068	2	0	9	0	1	3831.498	3	NC	1
383		2	max	.028	3	.05	3	.008	5	1.673e-4	3	NC	4	NC	1
384			min	-.031	2	-.041	2	0	9	-1.157e-5	9	1715.74	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.028	3	.02	3	.01	5	3.184e-4	3	NC	5	NC	1
386		min	-.031	2	-.015	2	0	9	-2.305e-5	9	879.473	2	NC	1
387	4	max	.028	3	.008	2	.012	5	3.054e-4	3	NC	5	NC	1
388		min	-.031	2	-.006	3	0	9	-2.199e-5	9	611.135	3	9817.767	3
389	5	max	.028	3	.028	2	.015	5	2.925e-4	5	NC	5	NC	1
390		min	-.031	2	-.027	3	0	9	-2.094e-5	9	477.414	3	8215.782	3
391	6	max	.028	3	.045	2	.018	5	3.045e-4	5	NC	5	NC	1
392		min	-.031	2	-.045	3	0	9	-1.988e-5	9	405.825	3	7421.778	3
393	7	max	.028	3	.059	2	.02	5	3.166e-4	5	NC	5	NC	1
394		min	-.031	2	-.058	3	0	9	-1.882e-5	9	364.103	3	7060.62	3
395	8	max	.028	3	.07	2	.024	5	3.286e-4	5	NC	5	NC	1
396		min	-.031	2	-.067	3	0	9	-1.777e-5	9	339.028	2	6988.476	3
397	9	max	.028	3	.077	2	.027	5	3.406e-4	5	NC	5	NC	1
398		min	-.031	2	-.072	3	0	9	-1.671e-5	9	323.008	2	7151.818	3
399	10	max	.028	3	.08	2	.03	5	3.527e-4	5	NC	5	NC	1
400		min	-.03	2	-.074	3	0	9	-1.566e-5	9	316.442	2	7545.696	3
401	11	max	.028	3	.079	2	.033	5	3.647e-4	5	NC	5	NC	1
402		min	-.03	2	-.071	3	0	9	-1.46e-5	9	318.67	2	8203.695	3
403	12	max	.028	3	.073	2	.036	4	3.767e-4	5	NC	5	NC	1
404		min	-.03	2	-.065	3	0	9	-1.355e-5	9	330.545	2	9205.092	3
405	13	max	.028	3	.064	2	.039	4	3.887e-4	5	NC	5	NC	1
406		min	-.03	2	-.056	3	0	9	-1.249e-5	9	354.912	2	NC	1
407	14	max	.027	3	.049	2	.041	4	4.008e-4	5	NC	5	NC	1
408		min	-.03	2	-.042	3	0	9	-1.143e-5	9	398.309	2	NC	1
409	15	max	.027	3	.03	2	.044	4	4.129e-4	4	NC	5	NC	1
410		min	-.03	2	-.026	3	0	9	-1.038e-5	9	476.188	2	NC	1
411	16	max	.027	3	.006	2	.046	4	5.974e-4	4	NC	5	NC	1
412		min	-.03	2	-.006	3	0	9	-1.028e-5	9	613.977	3	NC	1
413	17	max	.027	3	.018	3	.048	4	4.874e-3	4	NC	5	NC	1
414		min	-.03	2	-.024	2	0	9	-3.304e-5	9	890.596	3	NC	1
415	18	max	.027	3	.044	3	.05	4	2.503e-3	4	NC	4	NC	1
416		min	-.03	2	-.058	2	0	9	-1.692e-5	9	1746.395	3	NC	1
417	19	max	.027	3	.071	3	.051	4	5.986e-6	5	NC	3	NC	1
418		min	-.03	2	-.095	2	0	9	-2.321e-6	3	1794.861	2	NC	1
419	M9	1	max	.01	.026	3	.006	5	8.105e-3	3	NC	1	NC	1
420		min	-.01	2	-.022	2	0	9	-5.38e-3	2	NC	1	NC	1
421	2	max	.01	3	.016	3	.005	4	3.967e-3	3	NC	4	NC	1
422		min	-.01	2	-.013	2	0	10	-2.657e-3	2	5326.454	2	NC	1
423	3	max	.01	3	.006	3	.005	4	1.024e-4	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	10	-9.508e-5	3	2596.733	3	NC	1
425	4	max	.01	3	.002	2	.006	4	8.384e-5	1	NC	4	NC	1
426		min	-.01	2	-.003	3	-.001	3	-9.692e-5	3	1777.468	3	NC	1
427	5	max	.01	3	.009	2	.007	4	6.532e-5	1	NC	4	NC	1
428		min	-.01	2	-.01	3	-.002	3	-9.877e-5	3	1405.997	3	8081.935	3
429	6	max	.01	3	.015	2	.009	4	4.679e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.004	3	-1.006e-4	3	1202.881	3	7022.399	3
431	7	max	.01	3	.019	2	.011	4	2.827e-5	1	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-1.025e-4	3	1083.087	3	6409.468	3
433	8	max	.01	3	.022	2	.013	4	1.736e-5	4	NC	4	NC	1
434		min	-.01	2	-.022	3	-.005	3	-1.043e-4	3	1012.707	3	5883.87	4
435	9	max	.01	3	.025	2	.016	4	3.236e-5	5	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-1.061e-4	3	976.368	3	4376.303	4
437	10	max	.01	3	.026	2	.019	5	4.756e-5	5	NC	4	NC	1
438		min	-.01	2	-.025	3	-.005	3	-1.08e-4	3	967.19	3	3416.581	4
439	11	max	.01	3	.025	2	.023	5	6.277e-5	5	NC	4	NC	1
440		min	-.01	2	-.024	3	-.005	3	-1.098e-4	3	983.291	3	2766.244	4
441	12	max	.01	3	.023	2	.026	5	7.797e-5	5	NC	4	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442		min	-.01	2	-.022	3	-.005	3	-1.117e-4	3	1026.895	3	2294.006	5
443	13	max	.01	3	.02	2	.03	5	9.318e-5	5	NC	4	NC	1
444		min	-.01	2	-.019	3	-.005	3	-1.135e-4	3	1103.289	2	1947.731	5
445	14	max	.009	3	.016	2	.034	5	1.084e-4	5	NC	4	NC	1
446		min	-.01	2	-.014	3	-.004	3	-1.153e-4	3	1234.129	3	1689.915	5
447	15	max	.009	3	.01	2	.037	5	1.236e-4	5	NC	4	NC	1
448		min	-.01	2	-.009	3	-.003	3	-1.199e-4	1	1447.651	3	1493.266	5
449	16	max	.009	3	.002	2	.041	5	3.249e-4	5	NC	4	NC	1
450		min	-.01	2	-.002	3	-.003	3	-1.333e-4	1	1829.594	3	1340.496	5
451	17	max	.009	3	.006	3	.044	5	4.964e-3	4	NC	4	NC	1
452		min	-.01	2	-.008	2	-.002	1	-4.601e-5	9	2655.694	3	1220.116	5
453	18	max	.009	3	.015	3	.047	5	2.468e-3	5	NC	1	NC	1
454		min	-.01	2	-.019	2	-.001	9	4.005e-3	2	5208.43	3	1121.644	4
455	19	max	.009	3	.024	3	.051	4	4.62e-3	3	NC	1	NC	1
456		min	-.01	2	-.03	2	0	9	-8.082e-3	2	5643.596	2	1038.601	4
457	M13	1	max	0	.026	3	.01	3	3.992e-3	3	NC	1	NC	1
458		min	-.006	5	-.022	2	-.01	2	-3.344e-3	2	NC	1	NC	1
459	2	max	0	9	.065	3	.008	3	4.923e-3	3	NC	4	NC	1
460		min	-.006	5	-.048	2	-.009	2	-4.125e-3	2	2295.023	3	NC	1
461	3	max	0	9	.099	3	.008	3	5.855e-3	3	NC	4	NC	1
462		min	-.006	5	-.071	2	-.009	2	-4.907e-3	2	1241.027	3	NC	1
463	4	max	0	9	.122	3	.009	3	6.786e-3	3	NC	4	NC	2
464		min	-.006	5	-.087	2	-.01	2	-5.688e-3	2	938.789	3	8315.864	1
465	5	max	0	9	.134	3	.012	3	7.717e-3	3	NC	4	NC	2
466		min	-.006	5	-.096	2	-.013	2	-6.47e-3	2	839.216	3	8158.039	1
467	6	max	0	9	.133	3	.015	3	8.649e-3	3	NC	4	NC	1
468		min	-.006	5	-.097	2	-.016	2	-7.251e-3	2	843.79	3	9505.493	9
469	7	max	0	9	.123	3	.019	3	9.58e-3	3	NC	4	NC	1
470		min	-.006	5	-.091	2	-.021	2	-8.033e-3	2	935.806	3	8378.074	2
471	8	max	0	9	.106	3	.022	3	1.051e-2	3	NC	4	NC	1
472		min	-.006	5	-.082	2	-.025	2	-8.814e-3	2	1126.959	3	5864.453	2
473	9	max	0	9	.091	3	.025	3	1.144e-2	3	NC	4	NC	4
474		min	-.006	5	-.073	2	-.029	2	-9.596e-3	2	1409.836	3	4705.676	2
475	10	max	0	9	.083	3	.028	3	1.237e-2	3	NC	4	NC	4
476		min	-.006	5	-.068	2	-.031	2	-1.038e-2	2	1600.032	3	4338.52	2
477	11	max	0	9	.091	3	.03	3	1.145e-2	3	NC	4	NC	4
478		min	-.006	5	-.073	2	-.029	2	-9.596e-3	2	1409.834	3	4374.899	3
479	12	max	0	9	.107	3	.031	3	1.052e-2	3	NC	4	NC	1
480		min	-.006	5	-.082	2	-.025	2	-8.814e-3	2	1126.957	3	4321.088	3
481	13	max	0	9	.123	3	.029	3	9.591e-3	3	NC	4	NC	1
482		min	-.006	5	-.091	2	-.021	2	-8.033e-3	2	935.805	3	4586.34	3
483	14	max	0	9	.134	3	.027	3	8.663e-3	3	NC	4	NC	1
484		min	-.006	5	-.097	2	-.016	2	-7.251e-3	2	843.789	3	5226.204	3
485	15	max	0	9	.134	3	.024	3	7.735e-3	3	NC	4	NC	2
486		min	-.006	5	-.096	2	-.013	2	-6.47e-3	2	839.215	3	6458.998	3
487	16	max	0	9	.123	3	.02	3	6.807e-3	3	NC	4	NC	2
488		min	-.006	5	-.087	2	-.01	2	-5.688e-3	2	938.788	3	8315.88	1
489	17	max	0	9	.1	3	.016	3	5.879e-3	3	NC	4	NC	1
490		min	-.006	5	-.071	2	-.009	2	-4.907e-3	2	1241.026	3	NC	1
491	18	max	0	9	.066	3	.012	3	4.951e-3	3	NC	4	NC	1
492		min	-.006	5	-.048	2	-.009	2	-4.125e-3	2	2295.02	3	NC	1
493	19	max	0	9	.027	3	.01	3	4.023e-3	3	NC	1	NC	1
494		min	-.006	5	-.022	2	-.01	2	-3.344e-3	2	NC	1	NC	1
495	M16	1	max	0	.024	3	.009	3	4.416e-3	2	NC	1	NC	1
496		min	-.051	4	-.03	2	-.01	2	-3.427e-3	3	NC	1	NC	1
497	2	max	0	9	.046	3	.012	3	5.452e-3	2	NC	4	NC	1
498		min	-.051	4	-.07	2	-.009	2	-4.182e-3	3	2295.135	2	NC	1







**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
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Address:			
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E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag} ; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} ; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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- Refer to manufacturer's product literature for hole cleaning and installation instructions.