

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	100 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 15.70 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	
S_{DS} =	0.00	C_s = 0	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

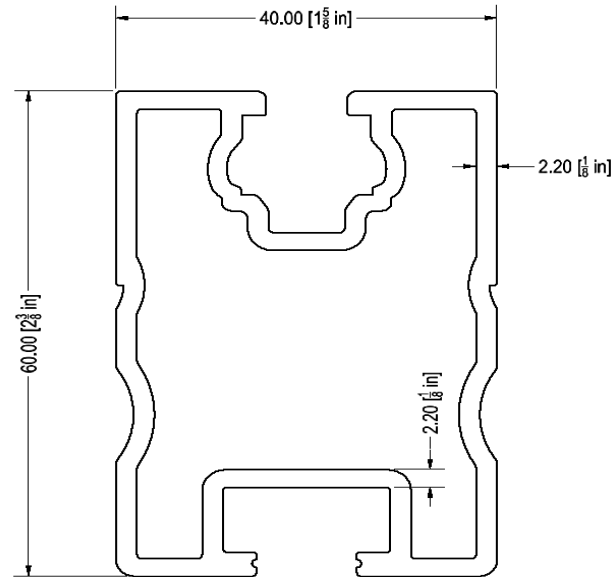
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	75 in
ΦF_{ty} STRONG-AXIS =	28.81 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.739 k-ft
M_z =	0.130 k-ft
$M_{y \text{ allowable}}$ =	1.226 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	75%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

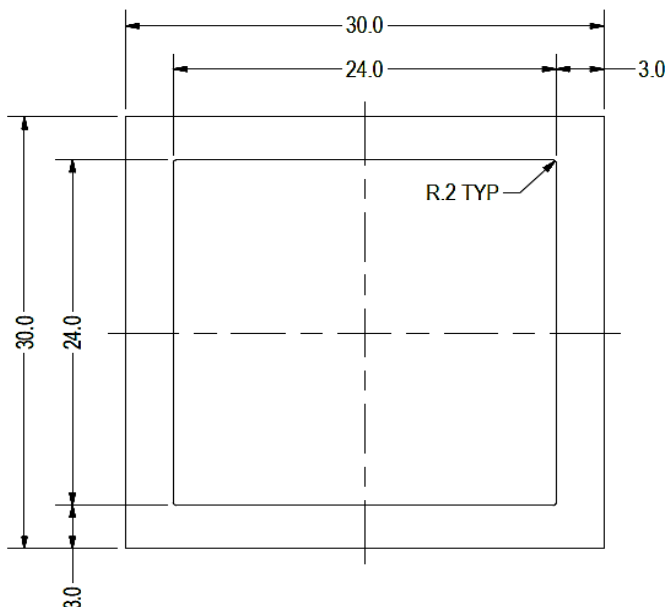
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.46 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.550 k-ft
M_z =	0.000 k-ft
P_n =	0.280 k
$M_{y \text{ allowable}}$ =	1.446 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	40%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

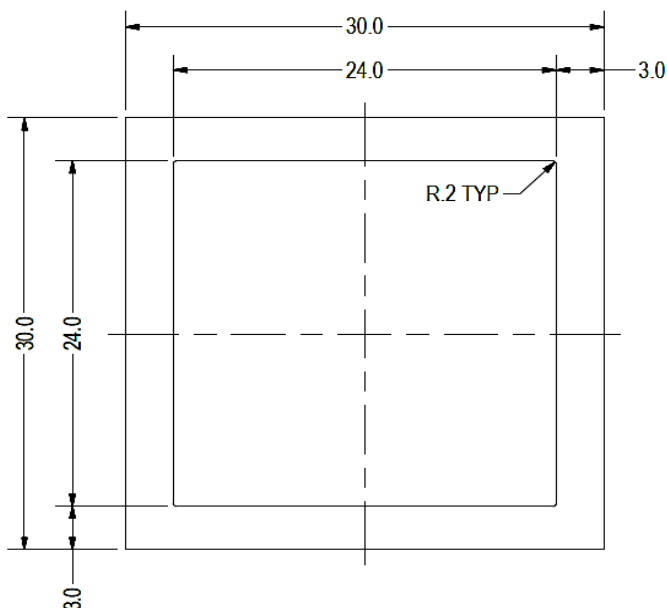
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.283 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	10%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.334 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.026 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.004 k-ft
P_n =	0.046 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	9%



A cross brace kit is required every 21 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

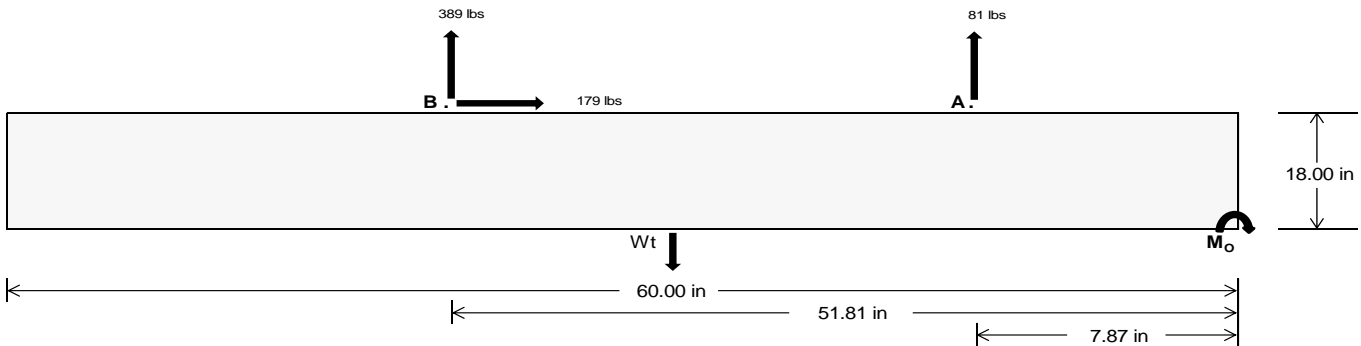
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	342.84	1621.75	k
Compressive Load =	1667.55	1301.07	k
Lateral Load =	2.81	745.16	k
Moment (Weak Axis) =	0.01	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24024.7$ in-lbs
Resisting Force Required = 800.82 lbs
S.F. = 1.67
Weight Required = 1334.71 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 179.08 lbs
Friction = 0.4
Weight Required = 447.70 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 179.08 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	598 lbs	598 lbs	598 lbs	598 lbs	543 lbs	543 lbs	543 lbs	543 lbs	811 lbs	811 lbs	811 lbs	811 lbs	-163 lbs	-163 lbs	-163 lbs	-163 lbs
F_B	436 lbs	436 lbs	436 lbs	436 lbs	478 lbs	478 lbs	478 lbs	478 lbs	653 lbs	653 lbs	653 lbs	653 lbs	-778 lbs	-778 lbs	-778 lbs	-778 lbs
F_V	52 lbs	52 lbs	52 lbs	52 lbs	320 lbs	320 lbs	320 lbs	320 lbs	275 lbs	275 lbs	275 lbs	275 lbs	-358 lbs	-358 lbs	-358 lbs	-358 lbs
P_{total}	3028 lbs	3118 lbs	3209 lbs	3300 lbs	3015 lbs	3105 lbs	3196 lbs	3287 lbs	3457 lbs	3547 lbs	3638 lbs	3729 lbs	255 lbs	310 lbs	364 lbs	418 lbs
M	386 lbs-ft	386 lbs-ft	386 lbs-ft	386 lbs-ft	610 lbs-ft	610 lbs-ft	610 lbs-ft	610 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	577 lbs-ft	577 lbs-ft	577 lbs-ft	577 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.26 ft	1.86 ft	1.59 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	279.7 psf	277.0 psf	274.5 psf	272.3 psf	249.0 psf	247.6 psf	246.3 psf	245.2 psf	282.7 psf	279.9 psf	277.3 psf	274.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	380.9 psf	373.8 psf	367.3 psf	361.3 psf	408.8 psf	400.5 psf	392.9 psf	385.8 psf	471.5 psf	460.5 psf	450.3 psf	441.0 psf	388.9 psf	169.5 psf	132.8 psf	119.5 psf

Maximum Bearing Pressure = 472 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

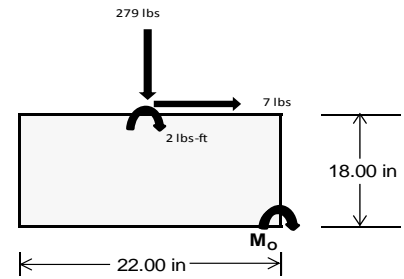
Overturning Check

$M_o = 242.4 \text{ ft-lbs}$
 Resisting Force Required = 264.39 lbs
 S.F. = 1.67
 Weight Required = 440.65 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	72 lbs	194 lbs	68 lbs	279 lbs	843 lbs	275 lbs	21 lbs	57 lbs	20 lbs
F_v	1 lbs	1 lbs	0 lbs	7 lbs	7 lbs	1 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2540 lbs	2662 lbs	2536 lbs	2628 lbs	3192 lbs	2624 lbs	743 lbs	778 lbs	742 lbs
M	2 lbs-ft	2 lbs-ft	0 lbs-ft	13 lbs-ft	10 lbs-ft	1 lbs-ft	1 lbs-ft	1 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.82 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	276.4 sqft	289.7 sqft	276.6 sqft	282.0 sqft	344.6 sqft	286.0 sqft	80.8 sqft	84.7 sqft	80.9 sqft
f_{max}	277.9 psf	291.1 psf	276.8 psf	291.4 psf	351.9 psf	286.6 psf	81.3 psf	85.1 psf	80.9 psf



Maximum Bearing Pressure = 352 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

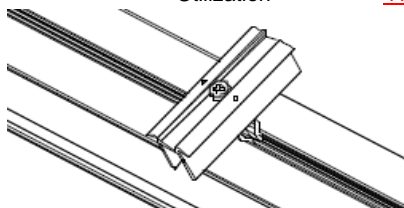
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

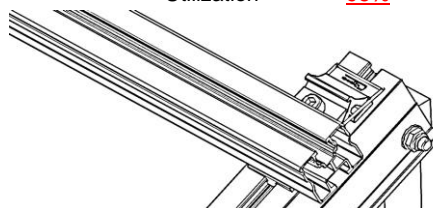
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.497 k
Allowable Uplift =	1.214 k
Utilization =	<u>41%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.096 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



6.2 Bolted Connections

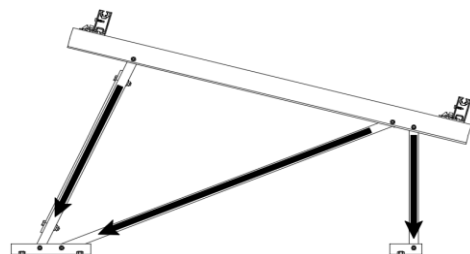
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.283 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>23%</u>

Diagonal Strut

Maximum Axial Load =	0.334 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.172 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.046 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.024 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.255$$

$$195.296$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 75.00 \text{ in}$$

$$J = 0.255$$

$$202.803$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.7$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.226 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1371 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.446 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	139.323	2	278.782	1	.008	11	0	1	0	1	0	1
2		min	-180.405	3	-379.273	3	-.127	3	0	3	0	1	0	1
3	N7	max	0	15	444.462	1	-.036	15	0	15	0	1	0	1
4		min	-.141	2	-72.808	3	-.935	1	-.002	1	0	1	0	1
5	N15	max	0	15	1282.728	1	.485	1	0	1	0	1	0	1
6		min	-1.535	2	-263.723	3	-.362	3	0	3	0	1	0	1
7	N16	max	531.583	2	1000.826	1	0	10	0	1	0	1	0	1
8		min	-573.201	3	-1247.497	3	-40.818	3	0	3	0	1	0	1
9	N23	max	0	15	444.235	1	2.161	1	.004	1	0	1	0	1
10		min	-.141	2	-72.398	3	.078	15	0	15	0	1	0	1
11	N24	max	139.59	2	283.401	1	41.159	3	.001	1	0	1	0	1
12		min	-180.545	3	-377.139	3	.008	10	0	3	0	1	0	1
13	Totals:	max	808.679	2	3734.435	1	0	1						
14		min	-934.467	3	-2412.838	3	0	10						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	315.486	1	.646	4	.506	1	0	15	0	3	0	1
2			min	-359.55	3	.153	15	-.073	3	0	1	0	1	0	1
3		2	max	315.593	1	.604	4	.506	1	0	15	0	1	0	15
4			min	-359.47	3	.143	15	-.073	3	0	1	0	10	0	4
5		3	max	315.699	1	.563	4	.506	1	0	15	0	1	0	15
6			min	-359.39	3	.133	15	-.073	3	0	1	0	10	0	4
7		4	max	315.806	1	.522	4	.506	1	0	15	0	1	0	15
8			min	-359.31	3	.123	15	-.073	3	0	1	0	3	0	4
9		5	max	315.913	1	.48	4	.506	1	0	15	0	1	0	15
10			min	-359.23	3	.114	15	-.073	3	0	1	0	3	0	4
11		6	max	316.019	1	.439	4	.506	1	0	15	0	1	0	15
12			min	-359.15	3	.104	15	-.073	3	0	1	0	3	0	4
13		7	max	316.126	1	.398	4	.506	1	0	15	0	1	0	15
14			min	-359.07	3	.094	15	-.073	3	0	1	0	3	0	4
15		8	max	316.232	1	.357	4	.506	1	0	15	0	1	0	15
16			min	-358.99	3	.085	15	-.073	3	0	1	0	3	0	4
17		9	max	316.339	1	.315	4	.506	1	0	15	0	1	0	15
18			min	-358.91	3	.075	15	-.073	3	0	1	0	3	0	4
19		10	max	316.445	1	.274	4	.506	1	0	15	0	1	0	15
20			min	-358.83	3	.065	15	-.073	3	0	1	0	3	0	4
21		11	max	316.552	1	.233	4	.506	1	0	15	0	1	0	15
22			min	-358.75	3	.056	15	-.073	3	0	1	0	3	0	4
23		12	max	316.658	1	.192	4	.506	1	0	15	0	1	0	15
24			min	-358.671	3	.046	15	-.073	3	0	1	0	3	0	4
25		13	max	316.765	1	.15	4	.506	1	0	15	0	1	0	15
26			min	-358.591	3	.036	15	-.073	3	0	1	0	3	0	4
27		14	max	316.871	1	.109	4	.506	1	0	15	0	1	0	15
28			min	-358.511	3	.026	15	-.073	3	0	1	0	3	0	4
29		15	max	316.978	1	.075	2	.506	1	0	15	.001	1	0	15
30			min	-358.431	3	.014	12	-.073	3	0	1	0	3	0	4
31		16	max	317.084	1	.043	2	.506	1	0	15	.001	1	0	15
32			min	-358.351	3	-.005	3	-.073	3	0	1	0	3	0	4
33		17	max	317.191	1	.011	2	.506	1	0	15	.001	1	0	15
34			min	-358.271	3	-.029	3	-.073	3	0	1	0	3	0	4
35		18	max	317.298	1	-.012	15	.506	1	0	15	.001	1	0	15
36			min	-358.191	3	-.056	4	-.073	3	0	1	0	3	0	4
37		19	max	317.404	1	-.022	15	.506	1	0	15	.001	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-358.111	3	-.097	4	-.073	3	0	1	0	3	0	4
39	M3	1	max	75.684	2	1.797	4	-.015	15	0	15	.001	1	0
40		min	-85.011	3	.423	15	-.436	1	0	1	0	15	0	15
41		2	max	75.616	2	1.619	4	-.015	15	0	15	.001	1	0
42		min	-85.062	3	.381	15	-.436	1	0	1	0	15	0	15
43		3	max	75.549	2	1.441	4	-.015	15	0	15	.001	1	0
44		min	-85.112	3	.339	15	-.436	1	0	1	0	15	0	3
45		4	max	75.481	2	1.264	4	-.015	15	0	15	.001	1	0
46		min	-85.163	3	.298	15	-.436	1	0	1	0	15	0	4
47		5	max	75.413	2	1.086	4	-.015	15	0	15	0	1	0
48		min	-85.214	3	.256	15	-.436	1	0	1	0	15	0	4
49		6	max	75.345	2	.908	4	-.015	15	0	15	0	1	0
50		min	-85.265	3	.214	15	-.436	1	0	1	0	15	0	4
51		7	max	75.277	2	.731	4	-.015	15	0	15	0	1	0
52		min	-85.316	3	.172	15	-.436	1	0	1	0	15	0	4
53		8	max	75.209	2	.553	4	-.015	15	0	15	0	1	0
54		min	-85.367	3	.131	15	-.436	1	0	1	0	15	0	4
55		9	max	75.141	2	.376	4	-.015	15	0	15	0	1	0
56		min	-85.418	3	.089	15	-.436	1	0	1	0	15	-.001	4
57		10	max	75.074	2	.198	4	-.015	15	0	15	0	1	0
58		min	-85.469	3	.047	15	-.436	1	0	1	0	15	-.001	4
59		11	max	75.006	2	.032	2	-.015	15	0	15	0	1	0
60		min	-85.52	3	-.003	3	-.436	1	0	1	0	15	-.001	4
61		12	max	74.938	2	-.036	15	-.015	15	0	15	0	1	0
62		min	-85.571	3	-.157	4	-.436	1	0	1	0	15	-.001	4
63		13	max	74.87	2	-.078	15	-.015	15	0	15	0	1	0
64		min	-85.621	3	-.335	4	-.436	1	0	1	0	15	-.001	4
65		14	max	74.802	2	-.12	15	-.015	15	0	15	0	1	0
66		min	-85.672	3	-.513	4	-.436	1	0	1	0	12	-.001	4
67		15	max	74.734	2	-.162	15	-.015	15	0	15	0	1	0
68		min	-85.723	3	-.69	4	-.436	1	0	1	0	3	0	4
69		16	max	74.666	2	-.203	15	-.015	15	0	15	0	15	0
70		min	-85.774	3	-.868	4	-.436	1	0	1	0	1	0	4
71		17	max	74.598	2	-.245	15	-.015	15	0	15	0	15	0
72		min	-85.825	3	-1.046	4	-.436	1	0	1	0	1	0	4
73		18	max	74.531	2	-.287	15	-.015	15	0	15	0	15	0
74		min	-85.876	3	-1.223	4	-.436	1	0	1	0	1	0	4
75		19	max	74.463	2	-.329	15	-.015	15	0	15	0	15	0
76		min	-85.927	3	-1.401	4	-.436	1	0	1	0	1	0	1
77	M4	1	max	443.297	1	0	1	-.036	15	0	1	0	3	0
78		min	-73.681	3	0	1	-1.011	1	0	1	0	1	0	1
79		2	max	443.362	1	0	1	-.036	15	0	1	0	12	0
80		min	-73.633	3	0	1	-1.011	1	0	1	0	1	0	1
81		3	max	443.427	1	0	1	-.036	15	0	1	0	15	0
82		min	-73.584	3	0	1	-1.011	1	0	1	0	1	0	1
83		4	max	443.491	1	0	1	-.036	15	0	1	0	15	0
84		min	-73.536	3	0	1	-1.011	1	0	1	0	1	0	1
85		5	max	443.556	1	0	1	-.036	15	0	1	0	15	0
86		min	-73.487	3	0	1	-1.011	1	0	1	0	1	0	1
87		6	max	443.621	1	0	1	-.036	15	0	1	0	15	0
88		min	-73.439	3	0	1	-1.011	1	0	1	0	1	0	1
89		7	max	443.685	1	0	1	-.036	15	0	1	0	15	0
90		min	-73.39	3	0	1	-1.011	1	0	1	0	1	0	1
91		8	max	443.75	1	0	1	-.036	15	0	1	0	15	0
92		min	-73.342	3	0	1	-1.011	1	0	1	0	1	0	1
93		9	max	443.815	1	0	1	-.036	15	0	1	0	15	0
94		min	-73.293	3	0	1	-1.011	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	443.88	1	0	1	-0.036	15	0	1	0	15	0	1
96			min	-73.245	3	0	1	-1.011	1	0	1	0	1	0	1
97		11	max	443.944	1	0	1	-0.036	15	0	1	0	15	0	1
98			min	-73.196	3	0	1	-1.011	1	0	1	0	1	0	1
99		12	max	444.009	1	0	1	-0.036	15	0	1	0	15	0	1
100			min	-73.148	3	0	1	-1.011	1	0	1	-.001	1	0	1
101		13	max	444.074	1	0	1	-0.036	15	0	1	0	15	0	1
102			min	-73.099	3	0	1	-1.011	1	0	1	-.001	1	0	1
103		14	max	444.138	1	0	1	-0.036	15	0	1	0	15	0	1
104			min	-73.051	3	0	1	-1.011	1	0	1	-.001	1	0	1
105		15	max	444.203	1	0	1	-0.036	15	0	1	0	15	0	1
106			min	-73.002	3	0	1	-1.011	1	0	1	-.001	1	0	1
107		16	max	444.268	1	0	1	-0.036	15	0	1	0	15	0	1
108			min	-72.953	3	0	1	-1.011	1	0	1	-.001	1	0	1
109		17	max	444.333	1	0	1	-0.036	15	0	1	0	15	0	1
110			min	-72.905	3	0	1	-1.011	1	0	1	-.001	1	0	1
111		18	max	444.397	1	0	1	-0.036	15	0	1	0	15	0	1
112			min	-72.856	3	0	1	-1.011	1	0	1	-.002	1	0	1
113		19	max	444.462	1	0	1	-0.036	15	0	1	0	15	0	1
114			min	-72.808	3	0	1	-1.011	1	0	1	-.002	1	0	1
115	M6	1	max	1023.778	1	.641	4	.236	1	0	1	0	3	0	1
116			min	-1172.304	3	.152	15	-.169	3	0	10	0	1	0	1
117		2	max	1023.885	1	.599	4	.236	1	0	1	0	3	0	15
118			min	-1172.224	3	.142	15	-.169	3	0	10	0	1	0	4
119		3	max	1023.991	1	.558	4	.236	1	0	1	0	3	0	15
120			min	-1172.144	3	.132	15	-.169	3	0	10	0	1	0	4
121		4	max	1024.098	1	.517	4	.236	1	0	1	0	11	0	15
122			min	-1172.064	3	.123	15	-.169	3	0	10	0	10	0	4
123		5	max	1024.204	1	.475	4	.236	1	0	1	0	1	0	15
124			min	-1171.984	3	.113	15	-.169	3	0	10	0	3	0	4
125		6	max	1024.311	1	.434	4	.236	1	0	1	0	1	0	15
126			min	-1171.905	3	.103	15	-.169	3	0	10	0	3	0	4
127		7	max	1024.417	1	.393	4	.236	1	0	1	0	1	0	15
128			min	-1171.825	3	.094	15	-.169	3	0	10	0	3	0	4
129		8	max	1024.524	1	.352	4	.236	1	0	1	0	1	0	15
130			min	-1171.745	3	.084	15	-.169	3	0	10	0	3	0	4
131		9	max	1024.63	1	.319	2	.236	1	0	1	0	1	0	15
132			min	-1171.665	3	.074	15	-.169	3	0	10	0	3	0	4
133		10	max	1024.737	1	.287	2	.236	1	0	1	0	1	0	15
134			min	-1171.585	3	.064	15	-.169	3	0	10	0	3	0	4
135		11	max	1024.843	1	.255	2	.236	1	0	1	0	1	0	15
136			min	-1171.505	3	.051	12	-.169	3	0	10	0	3	0	4
137		12	max	1024.95	1	.222	2	.236	1	0	1	0	1	0	15
138			min	-1171.425	3	.035	12	-.169	3	0	10	0	3	0	4
139		13	max	1025.057	1	.19	2	.236	1	0	1	0	1	0	15
140			min	-1171.345	3	.019	12	-.169	3	0	10	0	3	0	4
141		14	max	1025.163	1	.158	2	.236	1	0	1	0	1	0	15
142			min	-1171.265	3	0	3	-.169	3	0	10	0	3	0	4
143		15	max	1025.27	1	.126	2	.236	1	0	1	0	1	0	15
144			min	-1171.185	3	-.023	3	-.169	3	0	10	0	3	0	4
145		16	max	1025.376	1	.094	2	.236	1	0	1	0	1	0	15
146			min	-1171.105	3	-.047	3	-.169	3	0	10	0	3	0	2
147		17	max	1025.483	1	.062	2	.236	1	0	1	0	1	0	15
148			min	-1171.026	3	-.072	3	-.169	3	0	10	0	3	0	2
149		18	max	1025.589	1	.029	2	.236	1	0	1	0	1	0	15
150			min	-1170.946	3	-.096	3	-.169	3	0	10	0	3	0	2
151		19	max	1025.696	1	-.003	2	.236	1	0	1	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1170.866	3	-.12	3	-.169	3	0	10	0	3	0	2
153	M7	1	max	333.603	2	1.796	4	.007	1	0	1	0	1	2
154		min	-254.434	3	.423	15	-.007	2	0	3	0	3	0	12
155		2	max	333.535	2	1.618	4	.007	1	0	1	0	1	2
156		min	-254.484	3	.381	15	-.007	2	0	3	0	3	0	12
157		3	max	333.468	2	1.44	4	.007	1	0	1	0	1	2
158		min	-254.535	3	.339	15	-.007	2	0	3	0	3	0	3
159		4	max	333.4	2	1.263	4	.007	1	0	1	0	1	2
160		min	-254.586	3	.298	15	-.007	2	0	3	0	3	0	3
161		5	max	333.332	2	1.085	4	.007	1	0	1	0	1	15
162		min	-254.637	3	.256	15	-.007	2	0	3	0	3	0	4
163		6	max	333.264	2	.908	4	.007	1	0	1	0	1	15
164		min	-254.688	3	.214	15	-.007	2	0	3	0	3	0	4
165		7	max	333.196	2	.73	4	.007	1	0	1	0	1	15
166		min	-254.739	3	.172	15	-.007	2	0	3	0	3	0	4
167		8	max	333.128	2	.552	4	.007	1	0	1	0	1	15
168		min	-254.79	3	.131	15	-.007	2	0	3	0	3	0	4
169		9	max	333.06	2	.375	4	.007	1	0	1	0	1	15
170		min	-254.841	3	.089	15	-.007	2	0	3	0	3	-.001	4
171		10	max	332.993	2	.217	2	.007	1	0	1	0	1	15
172		min	-254.892	3	.043	12	-.007	2	0	3	0	3	-.001	4
173		11	max	332.925	2	.078	2	.007	1	0	1	0	1	15
174		min	-254.943	3	-.042	3	-.007	2	0	3	0	3	-.001	4
175		12	max	332.857	2	-.036	15	.007	1	0	1	0	1	15
176		min	-254.993	3	-.158	4	-.007	2	0	3	0	3	-.001	4
177		13	max	332.789	2	-.078	15	.007	1	0	1	0	1	15
178		min	-255.044	3	-.336	4	-.007	2	0	3	0	3	-.001	4
179		14	max	332.721	2	-.12	15	.007	1	0	1	0	1	15
180		min	-255.095	3	-.514	4	-.007	2	0	3	0	3	-.001	4
181		15	max	332.653	2	-.162	15	.007	1	0	1	0	1	15
182		min	-255.146	3	-.691	4	-.007	2	0	3	0	3	0	4
183		16	max	332.585	2	-.204	15	.007	1	0	1	0	1	15
184		min	-255.197	3	-.869	4	-.007	2	0	3	0	3	0	4
185		17	max	332.517	2	-.245	15	.007	1	0	1	0	1	15
186		min	-255.248	3	-1.047	4	-.007	2	0	3	0	3	0	4
187		18	max	332.45	2	-.287	15	.007	1	0	1	0	1	15
188		min	-255.299	3	-1.224	4	-.007	2	0	3	0	3	0	4
189		19	max	332.382	2	-.329	15	.007	1	0	1	0	1	1
190		min	-255.35	3	-1.402	4	-.007	2	0	3	0	3	0	1
191	M8	1	max	1281.563	1	0	1	.613	1	0	1	0	10	1
192		min	-264.596	3	0	1	-.347	3	0	1	0	1	0	1
193		2	max	1281.628	1	0	1	.613	1	0	1	0	1	1
194		min	-264.548	3	0	1	-.347	3	0	1	0	3	0	1
195		3	max	1281.693	1	0	1	.613	1	0	1	0	1	1
196		min	-264.499	3	0	1	-.347	3	0	1	0	3	0	1
197		4	max	1281.757	1	0	1	.613	1	0	1	0	1	1
198		min	-264.451	3	0	1	-.347	3	0	1	0	3	0	1
199		5	max	1281.822	1	0	1	.613	1	0	1	0	1	1
200		min	-264.402	3	0	1	-.347	3	0	1	0	3	0	1
201		6	max	1281.887	1	0	1	.613	1	0	1	0	1	1
202		min	-264.354	3	0	1	-.347	3	0	1	0	3	0	1
203		7	max	1281.952	1	0	1	.613	1	0	1	0	1	1
204		min	-264.305	3	0	1	-.347	3	0	1	0	3	0	1
205		8	max	1282.016	1	0	1	.613	1	0	1	0	1	1
206		min	-264.257	3	0	1	-.347	3	0	1	0	3	0	1
207		9	max	1282.081	1	0	1	.613	1	0	1	0	1	1
208		min	-264.208	3	0	1	-.347	3	0	1	0	3	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209		10	max	1282.146	1	0	1	.613	1	0	1	0	1	0	1
210			min	-264.16	3	0	1	-.347	3	0	1	0	3	0	1
211		11	max	1282.21	1	0	1	.613	1	0	1	0	1	0	1
212			min	-264.111	3	0	1	-.347	3	0	1	0	3	0	1
213		12	max	1282.275	1	0	1	.613	1	0	1	0	1	0	1
214			min	-264.063	3	0	1	-.347	3	0	1	0	3	0	1
215		13	max	1282.34	1	0	1	.613	1	0	1	0	1	0	1
216			min	-264.014	3	0	1	-.347	3	0	1	0	3	0	1
217		14	max	1282.404	1	0	1	.613	1	0	1	0	1	0	1
218			min	-263.965	3	0	1	-.347	3	0	1	0	3	0	1
219		15	max	1282.469	1	0	1	.613	1	0	1	0	1	0	1
220			min	-263.917	3	0	1	-.347	3	0	1	0	3	0	1
221		16	max	1282.534	1	0	1	.613	1	0	1	0	1	0	1
222			min	-263.868	3	0	1	-.347	3	0	1	0	3	0	1
223		17	max	1282.599	1	0	1	.613	1	0	1	0	1	0	1
224			min	-263.82	3	0	1	-.347	3	0	1	0	3	0	1
225		18	max	1282.663	1	0	1	.613	1	0	1	0	1	0	1
226			min	-263.771	3	0	1	-.347	3	0	1	0	3	0	1
227		19	max	1282.728	1	0	1	.613	1	0	1	0	1	0	1
228			min	-263.723	3	0	1	-.347	3	0	1	0	3	0	1
229	M10	1	max	319.514	1	.637	4	-.003	15	0	1	0	1	0	1
230			min	-340.203	3	.151	15	-.088	1	0	3	0	3	0	1
231		2	max	319.62	1	.596	4	-.003	15	0	1	0	1	0	15
232			min	-340.123	3	.142	15	-.088	1	0	3	0	3	0	4
233		3	max	319.727	1	.554	4	-.003	15	0	1	0	1	0	15
234			min	-340.043	3	.132	15	-.088	1	0	3	0	3	0	4
235		4	max	319.834	1	.513	4	-.003	15	0	1	0	1	0	15
236			min	-339.964	3	.122	15	-.088	1	0	3	0	3	0	4
237		5	max	319.94	1	.472	4	-.003	15	0	1	0	1	0	15
238			min	-339.884	3	.112	15	-.088	1	0	3	0	3	0	4
239		6	max	320.047	1	.43	4	-.003	15	0	1	0	1	0	15
240			min	-339.804	3	.103	15	-.088	1	0	3	0	3	0	4
241		7	max	320.153	1	.389	4	-.003	15	0	1	0	1	0	15
242			min	-339.724	3	.093	15	-.088	1	0	3	0	3	0	4
243		8	max	320.26	1	.348	4	-.003	15	0	1	0	1	0	15
244			min	-339.644	3	.083	15	-.088	1	0	3	0	3	0	4
245		9	max	320.366	1	.307	4	-.003	15	0	1	0	1	0	15
246			min	-339.564	3	.074	15	-.088	1	0	3	0	3	0	4
247		10	max	320.473	1	.265	4	-.003	15	0	1	0	15	0	15
248			min	-339.484	3	.064	15	-.088	1	0	3	0	3	0	4
249		11	max	320.579	1	.224	4	-.003	15	0	1	0	15	0	15
250			min	-339.404	3	.054	15	-.088	1	0	3	0	3	0	4
251		12	max	320.686	1	.183	4	-.003	15	0	1	0	15	0	15
252			min	-339.324	3	.045	15	-.088	1	0	3	0	3	0	4
253		13	max	320.792	1	.142	4	-.003	15	0	1	0	15	0	15
254			min	-339.244	3	.035	15	-.088	1	0	3	0	3	0	4
255		14	max	320.899	1	.107	2	-.003	15	0	1	0	15	0	15
256			min	-339.164	3	.025	15	-.088	1	0	3	0	3	0	4
257		15	max	321.006	1	.075	2	-.003	15	0	1	0	15	0	15
258			min	-339.085	3	-.003	1	-.088	1	0	3	0	3	0	4
259		16	max	321.112	1	.043	2	-.003	15	0	1	0	15	0	15
260			min	-339.005	3	-.035	1	-.088	1	0	3	0	3	0	4
261		17	max	321.219	1	.011	2	-.003	15	0	1	0	15	0	15
262			min	-338.925	3	-.068	1	-.088	1	0	3	0	3	0	4
263		18	max	321.325	1	-.011	12	-.003	15	0	1	0	15	0	15
264			min	-338.845	3	-.1	1	-.088	1	0	3	0	3	0	4
265		19	max	321.432	1	-.023	15	-.003	15	0	1	0	15	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-338.765	3	-.132	1	-.088	1	0	3	0	3	0	4
267	M11	1	max	75.343	2	1.801	4	.535	1	0	1	0	3	0	4
268			min	-85.614	3	.424	15	0	3	0	15	-.001	1	0	15
269		2	max	75.276	2	1.624	4	.535	1	0	1	0	3	0	4
270			min	-85.665	3	.382	15	0	3	0	15	-.001	1	0	12
271		3	max	75.208	2	1.446	4	.535	1	0	1	0	3	0	2
272			min	-85.716	3	.34	15	0	3	0	15	-.001	1	0	3
273		4	max	75.14	2	1.268	4	.535	1	0	1	0	3	0	15
274			min	-85.767	3	.298	15	0	3	0	15	-.001	1	0	3
275		5	max	75.072	2	1.091	4	.535	1	0	1	0	3	0	15
276			min	-85.817	3	.257	15	0	3	0	15	-.001	1	0	4
277		6	max	75.004	2	.913	4	.535	1	0	1	0	3	0	15
278			min	-85.868	3	.215	15	0	3	0	15	0	1	0	4
279		7	max	74.936	2	.735	4	.535	1	0	1	0	3	0	15
280			min	-85.919	3	.173	15	0	3	0	15	0	1	0	4
281		8	max	74.868	2	.558	4	.535	1	0	1	0	3	0	15
282			min	-85.97	3	.131	15	0	3	0	15	0	1	0	4
283		9	max	74.801	2	.38	4	.535	1	0	1	0	3	0	15
284			min	-86.021	3	.089	15	0	3	0	15	0	1	-.001	4
285		10	max	74.733	2	.202	4	.535	1	0	1	0	3	0	15
286			min	-86.072	3	.048	15	0	3	0	15	0	1	-.001	4
287		11	max	74.665	2	.032	2	.535	1	0	1	0	3	0	15
288			min	-86.123	3	-.022	3	0	3	0	15	0	1	-.001	4
289		12	max	74.597	2	-.036	15	.535	1	0	1	0	3	0	15
290			min	-86.174	3	-.153	4	0	3	0	15	0	1	-.001	4
291		13	max	74.529	2	-.078	15	.535	1	0	1	0	3	0	15
292			min	-86.225	3	-.331	4	0	3	0	15	0	1	-.001	4
293		14	max	74.461	2	-.119	15	.535	1	0	1	0	3	0	15
294			min	-86.276	3	-.508	4	0	3	0	15	0	10	-.001	4
295		15	max	74.393	2	-.161	15	.535	1	0	1	0	3	0	15
296			min	-86.326	3	-.686	4	0	3	0	15	0	10	0	4
297		16	max	74.326	2	-.203	15	.535	1	0	1	0	1	0	15
298			min	-86.377	3	-.864	4	0	3	0	15	0	15	0	4
299		17	max	74.258	2	-.245	15	.535	1	0	1	0	1	0	15
300			min	-86.428	3	-1.041	4	0	3	0	15	0	15	0	4
301		18	max	74.19	2	-.286	15	.535	1	0	1	0	1	0	15
302			min	-86.479	3	-1.219	4	0	3	0	15	0	15	0	4
303		19	max	74.122	2	-.328	15	.535	1	0	1	0	1	0	1
304			min	-86.53	3	-1.397	4	0	3	0	15	0	15	0	1
305	M12	1	max	443.071	1	0	1	2.332	1	0	1	0	1	0	1
306			min	-73.272	3	0	1	.078	15	0	1	0	3	0	1
307		2	max	443.135	1	0	1	2.332	1	0	1	0	1	0	1
308			min	-73.223	3	0	1	.078	15	0	1	0	15	0	1
309		3	max	443.2	1	0	1	2.332	1	0	1	0	1	0	1
310			min	-73.175	3	0	1	.078	15	0	1	0	15	0	1
311		4	max	443.265	1	0	1	2.332	1	0	1	0	1	0	1
312			min	-73.126	3	0	1	.078	15	0	1	0	15	0	1
313		5	max	443.329	1	0	1	2.332	1	0	1	0	1	0	1
314			min	-73.078	3	0	1	.078	15	0	1	0	15	0	1
315		6	max	443.394	1	0	1	2.332	1	0	1	.001	1	0	1
316			min	-73.029	3	0	1	.078	15	0	1	0	15	0	1
317		7	max	443.459	1	0	1	2.332	1	0	1	.001	1	0	1
318			min	-72.981	3	0	1	.078	15	0	1	0	15	0	1
319		8	max	443.524	1	0	1	2.332	1	0	1	.001	1	0	1
320			min	-72.932	3	0	1	.078	15	0	1	0	15	0	1
321		9	max	443.588	1	0	1	2.332	1	0	1	.002	1	0	1
322			min	-72.884	3	0	1	.078	15	0	1	0	15	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	443.653	1	0	1	2.332	1	0	1	.002	1	0	1
324		min	-72.835	3	0	1	.078	15	0	1	0	15	0	1
325	11	max	443.718	1	0	1	2.332	1	0	1	.002	1	0	1
326		min	-72.787	3	0	1	.078	15	0	1	0	15	0	1
327	12	max	443.782	1	0	1	2.332	1	0	1	.002	1	0	1
328		min	-72.738	3	0	1	.078	15	0	1	0	15	0	1
329	13	max	443.847	1	0	1	2.332	1	0	1	.003	1	0	1
330		min	-72.69	3	0	1	.078	15	0	1	0	15	0	1
331	14	max	443.912	1	0	1	2.332	1	0	1	.003	1	0	1
332		min	-72.641	3	0	1	.078	15	0	1	0	15	0	1
333	15	max	443.976	1	0	1	2.332	1	0	1	.003	1	0	1
334		min	-72.592	3	0	1	.078	15	0	1	0	15	0	1
335	16	max	444.041	1	0	1	2.332	1	0	1	.003	1	0	1
336		min	-72.544	3	0	1	.078	15	0	1	0	15	0	1
337	17	max	444.106	1	0	1	2.332	1	0	1	.003	1	0	1
338		min	-72.495	3	0	1	.078	15	0	1	0	15	0	1
339	18	max	444.171	1	0	1	2.332	1	0	1	.004	1	0	1
340		min	-72.447	3	0	1	.078	15	0	1	0	15	0	1
341	19	max	444.235	1	0	1	2.332	1	0	1	.004	1	0	1
342		min	-72.398	3	0	1	.078	15	0	1	0	15	0	1
343	M1	1	max	104.435	1	339.402	3	-1.563	15	0	.091	1	0	1
344		min	3.475	15	-316.57	1	-46.293	1	0	3	.003	15	0	3
345	2	max	104.53	1	339.205	3	-1.563	15	0	1	.081	1	.069	1
346		min	3.504	15	-316.833	1	-46.293	1	0	3	.003	15	-.074	3
347	3	max	87.439	1	5.945	9	-1.547	15	0	12	.07	1	.136	1
348		min	2.838	10	-19.981	3	-46.026	1	0	1	.002	15	-.146	3
349	4	max	87.535	1	5.726	9	-1.547	15	0	12	.06	1	.137	1
350		min	2.917	10	-20.177	3	-46.026	1	0	1	.002	15	-.142	3
351	5	max	87.63	1	5.508	9	-1.547	15	0	12	.05	1	.137	1
352		min	2.997	10	-20.374	3	-46.026	1	0	1	.002	15	-.137	3
353	6	max	87.726	1	5.289	9	-1.547	15	0	12	.04	1	.138	1
354		min	3.076	10	-20.571	3	-46.026	1	0	1	.001	15	-.133	3
355	7	max	87.821	1	5.07	9	-1.547	15	0	12	.03	1	.138	1
356		min	3.156	10	-20.768	3	-46.026	1	0	1	.001	15	-.128	3
357	8	max	87.917	1	4.852	9	-1.547	15	0	12	.02	1	.139	1
358		min	3.236	10	-20.965	3	-46.026	1	0	1	0	15	-.124	3
359	9	max	88.012	1	4.633	9	-1.547	15	0	12	.01	1	.14	1
360		min	3.315	10	-21.161	3	-46.026	1	0	1	0	15	-.119	3
361	10	max	88.108	1	4.414	9	-1.547	15	0	12	0	3	.14	1
362		min	3.395	10	-21.358	3	-46.026	1	0	1	0	15	-.115	3
363	11	max	88.203	1	4.196	9	-1.547	15	0	12	0	3	.143	2
364		min	3.437	15	-21.555	3	-46.026	1	0	1	-.01	1	-.11	3
365	12	max	88.299	1	3.977	9	-1.547	15	0	12	0	12	.147	2
366		min	3.466	15	-21.752	3	-46.026	1	0	1	-.02	1	-.105	3
367	13	max	88.394	1	3.758	9	-1.547	15	0	12	0	12	.151	2
368		min	3.495	15	-21.949	3	-46.026	1	0	1	-.03	1	-.1	3
369	14	max	88.49	1	3.54	9	-1.547	15	0	12	-.001	15	.155	2
370		min	3.523	15	-22.145	3	-46.026	1	0	1	-.04	1	-.096	3
371	15	max	88.585	1	3.321	9	-1.547	15	0	12	-.002	15	.16	2
372		min	3.552	15	-22.342	3	-46.026	1	0	1	-.05	1	-.091	3
373	16	max	80.1	2	33.66	2	-1.562	15	0	1	-.002	15	.163	2
374		min	-30.916	3	-84.97	3	-46.412	1	0	12	-.06	1	-.085	3
375	17	max	80.195	2	33.398	2	-1.562	15	0	1	-.002	15	.157	1
376		min	-30.844	3	-85.167	3	-46.412	1	0	12	-.07	1	-.067	3
377	18	max	-3.496	15	365.672	1	-1.598	15	0	3	-.003	15	.08	1
378		min	-104.425	1	-154.669	3	-47.478	1	0	1	-.081	1	-.034	3
379	19	max	-3.467	15	365.409	1	-1.598	15	0	3	-.003	15	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380			min	-104.33	1	-154.865	3	-47.478	1	0	1	-.091	1	0	3
381	M5	1	max	233.307	1	1119.431	3	0	10	0	1	.004	3	0	3
382			min	5.698	12	-1044.151	1	-36.724	3	0	3	0	10	0	1
383		2	max	233.402	1	1119.234	3	0	10	0	1	0	1	.226	1
384			min	5.746	12	-1044.414	1	-36.724	3	0	3	-.004	3	-.242	3
385		3	max	176.832	1	7.388	9	4.191	3	0	3	0	11	.448	1
386			min	1.752	10	-71.012	3	-.388	1	0	1	-.011	3	-.48	3
387		4	max	176.928	1	7.17	9	4.191	3	0	3	0	11	.453	1
388			min	1.831	10	-71.209	3	-.388	1	0	1	-.01	3	-.465	3
389		5	max	177.023	1	6.951	9	4.191	3	0	3	0	11	.458	1
390			min	1.911	10	-71.405	3	-.388	1	0	1	-.009	3	-.449	3
391		6	max	177.119	1	6.732	9	4.191	3	0	3	0	11	.463	1
392			min	1.991	10	-71.602	3	-.388	1	0	1	-.009	3	-.434	3
393		7	max	177.214	1	6.514	9	4.191	3	0	3	0	11	.468	1
394			min	2.07	10	-71.799	3	-.388	1	0	1	-.008	3	-.418	3
395		8	max	177.31	1	6.295	9	4.191	3	0	3	0	11	.473	1
396			min	2.15	10	-71.996	3	-.388	1	0	1	-.007	3	-.402	3
397		9	max	177.405	1	6.076	9	4.191	3	0	3	0	11	.479	1
398			min	2.229	10	-72.193	3	-.388	1	0	1	-.006	3	-.387	3
399		10	max	177.501	1	5.858	9	4.191	3	0	3	0	10	.484	1
400			min	2.309	10	-72.389	3	-.388	1	0	1	-.005	3	-.371	3
401		11	max	177.596	1	5.639	9	4.191	3	0	3	0	10	.489	1
402			min	2.389	10	-72.586	3	-.388	1	0	1	-.004	3	-.355	3
403		12	max	177.692	1	5.42	9	4.191	3	0	3	0	10	.496	2
404			min	2.468	10	-72.783	3	-.388	1	0	1	-.003	3	-.34	3
405		13	max	177.787	1	5.202	9	4.191	3	0	3	0	10	.51	2
406			min	2.548	10	-72.98	3	-.388	1	0	1	-.002	3	-.324	3
407		14	max	177.883	1	4.983	9	4.191	3	0	3	0	10	.524	2
408			min	2.627	10	-73.177	3	-.388	1	0	1	-.001	3	-.308	3
409		15	max	177.978	1	4.764	9	4.191	3	0	3	0	10	.538	2
410			min	2.707	10	-73.373	3	-.388	1	0	1	0	1	-.292	3
411		16	max	280.147	2	176.603	2	4.16	3	0	1	0	3	.55	2
412			min	-100.302	3	-257.408	3	-.379	1	0	10	0	1	-.275	3
413		17	max	280.243	2	176.341	2	4.16	3	0	1	.001	3	.516	1
414			min	-100.231	3	-257.605	3	-.379	1	0	10	0	1	-.219	3
415		18	max	-6.666	12	1201.513	1	3.814	3	0	3	.002	3	.26	1
416			min	-233.544	1	-507.222	3	-.072	11	0	1	0	1	-.11	3
417		19	max	-6.618	12	1201.251	1	3.814	3	0	3	.003	3	0	3
418			min	-233.448	1	-507.418	3	-.072	11	0	1	0	1	0	1
419	M9	1	max	104.016	1	339.376	3	49.776	1	0	3	-.003	15	0	1
420			min	3.46	15	-316.566	1	1.875	15	0	1	-.09	1	0	3
421		2	max	104.111	1	339.18	3	49.776	1	0	3	-.001	12	.069	1
422			min	3.488	15	-316.828	1	1.875	15	0	1	-.079	1	-.074	3
423		3	max	87.487	1	5.922	9	44.574	1	0	1	.007	3	.136	1
424			min	3.281	10	-19.916	3	-.347	3	0	15	-.067	1	-.146	3
425		4	max	87.582	1	5.703	9	44.574	1	0	1	.007	3	.137	1
426			min	3.361	10	-20.112	3	-.347	3	0	15	-.058	1	-.141	3
427		5	max	87.678	1	5.485	9	44.574	1	0	1	.006	3	.137	1
428			min	3.407	15	-20.309	3	-.347	3	0	15	-.048	1	-.137	3
429		6	max	87.773	1	5.266	9	44.574	1	0	1	.006	3	.138	1
430			min	3.436	15	-20.506	3	-.347	3	0	15	-.038	1	-.133	3
431		7	max	87.869	1	5.047	9	44.574	1	0	1	.006	3	.138	1
432			min	3.465	15	-20.703	3	-.347	3	0	15	-.029	1	-.128	3
433		8	max	87.964	1	4.829	9	44.574	1	0	1	.006	3	.139	1
434			min	3.494	15	-20.9	3	-.347	3	0	15	-.019	1	-.124	3
435		9	max	88.06	1	4.61	9	44.574	1	0	1	.006	3	.14	1
436			min	3.523	15	-21.096	3	-.347	3	0	15	-.009	1	-.119	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437	10	max	88.155	1	4.391	9	44.574	1	0	1	.006	3	.14	1
438		min	3.551	15	-21.293	3	-.347	3	0	15	0	2	-.115	3
439	11	max	88.251	1	4.173	9	44.574	1	0	1	.01	1	.143	2
440		min	3.58	15	-21.49	3	-.347	3	0	15	0	15	-.11	3
441	12	max	88.346	1	3.954	9	44.574	1	0	1	.02	1	.147	2
442		min	3.609	15	-21.687	3	-.347	3	0	15	0	15	-.105	3
443	13	max	88.442	1	3.735	9	44.574	1	0	1	.029	1	.151	2
444		min	3.638	15	-21.884	3	-.347	3	0	15	.001	15	-.1	3
445	14	max	88.537	1	3.517	9	44.574	1	0	1	.039	1	.155	2
446		min	3.667	15	-22.08	3	-.347	3	0	15	.001	15	-.096	3
447	15	max	88.633	1	3.298	9	44.574	1	0	1	.049	1	.159	2
448		min	3.695	15	-22.277	3	-.347	3	0	15	.002	15	-.091	3
449	16	max	80.287	2	33.404	2	45.07	1	0	15	.059	1	.163	2
450		min	-31.156	3	-85.352	3	-.338	3	0	1	.002	15	-.086	3
451	17	max	80.382	2	33.141	2	45.07	1	0	15	.069	1	.157	1
452		min	-31.085	3	-85.549	3	-.338	3	0	1	.002	15	-.067	3
453	18	max	-3.485	15	365.673	1	47.377	1	0	2	.079	1	.08	1
454		min	-104.075	1	-154.666	3	.017	3	0	3	.003	15	-.034	3
455	19	max	-3.457	15	365.411	1	47.377	1	0	2	.09	1	0	1
456		min	-103.979	1	-154.862	3	.017	3	0	3	.003	15	0	3
457	M13	1	max	49.894	1	316.203	1	-3.46	15	0	.09	1	0	1
458		min	1.875	15	-339.38	3	-104.006	1	0	3	.003	15	0	3
459	2	max	49.894	1	223.232	1	-2.648	15	0	1	.026	1	.201	3
460		min	1.875	15	-239.488	3	-79.464	1	0	3	0	15	-.187	1
461	3	max	49.894	1	130.262	1	-1.836	15	0	1	.003	3	.333	3
462		min	1.875	15	-139.595	3	-54.922	1	0	3	-.021	1	-.31	1
463	4	max	49.894	1	37.291	1	-1.024	15	0	1	0	3	.395	3
464		min	1.875	15	-39.703	3	-30.38	1	0	3	-.05	1	-.368	1
465	5	max	49.894	1	60.19	3	.033	10	0	1	0	12	.388	3
466		min	1.875	15	-55.679	1	-5.838	1	0	3	-.063	1	-.362	1
467	6	max	49.894	1	160.082	3	18.704	1	0	1	0	12	.311	3
468		min	1.875	15	-148.65	1	-.401	3	0	3	-.058	1	-.291	1
469	7	max	49.894	1	259.975	3	43.246	1	0	1	0	12	.165	3
470		min	1.875	15	-241.621	1	.573	12	0	3	-.037	1	-.155	1
471	8	max	49.894	1	359.867	3	67.789	1	0	1	.002	2	.045	1
472		min	1.875	15	-334.591	1	1.361	12	0	3	0	3	-.05	3
473	9	max	49.894	1	459.76	3	92.331	1	0	1	.057	1	.309	1
474		min	1.875	15	-427.562	1	2.149	12	0	3	.001	12	-.334	3
475	10	max	49.894	1	559.652	3	116.873	1	0	1	.13	1	.639	1
476		min	1.875	15	-520.532	1	2.936	12	0	3	.003	12	-.688	3
477	11	max	46.407	1	427.562	1	-1.971	12	0	3	.056	1	.309	1
478		min	1.563	15	-459.76	3	-91.911	1	0	1	-.002	3	-.334	3
479	12	max	46.407	1	334.591	1	-1.183	12	0	3	.002	2	.045	1
480		min	1.563	15	-359.867	3	-67.369	1	0	1	-.004	3	-.05	3
481	13	max	46.407	1	241.62	1	-.395	12	0	3	-.001	15	.165	3
482		min	1.563	15	-259.975	3	-42.827	1	0	1	-.037	1	-.155	1
483	14	max	46.407	1	148.65	1	.683	3	0	3	-.002	15	.311	3
484		min	1.563	15	-160.082	3	-18.285	1	0	1	-.059	1	-.291	1
485	15	max	46.407	1	55.679	1	6.257	1	0	3	-.002	15	.388	3
486		min	1.563	15	-60.19	3	-.033	10	0	1	-.063	1	-.362	1
487	16	max	46.407	1	39.703	3	30.799	1	0	3	-.001	12	.395	3
488		min	1.563	15	-37.291	1	1.04	15	0	1	-.05	1	-.368	1
489	17	max	46.407	1	139.595	3	55.342	1	0	3	0	3	.333	3
490		min	1.563	15	-130.262	1	1.852	15	0	1	-.02	1	-.31	1
491	18	max	46.407	1	239.488	3	79.884	1	0	3	.027	1	.201	3
492		min	1.563	15	-223.233	1	2.664	15	0	1	0	15	-.187	1
493	19	max	46.407	1	339.38	3	104.426	1	0	3	.091	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494		min	1.563	15	-316.203	1	3.475	15	0	1	.003	15	0	3
495	M16	1	max	3	365.794	1	-3.457	15	0	3	.09	1	0	2
496		min	-47.255	1	-154.879	3	-103.989	1	0	1	.003	15	0	3
497		2	max	3	258.229	1	-2.645	15	0	3	.026	1	.092	3
498		min	-47.255	1	-109.475	3	-79.447	1	0	1	0	15	-.217	1
499		3	max	3	150.663	1	-1.833	15	0	3	0	12	.152	3
500		min	-47.255	1	-64.071	3	-54.905	1	0	1	-.021	1	-.359	1
501		4	max	3	43.098	1	-1.021	15	0	3	-.002	15	.181	3
502		min	-47.255	1	-18.668	3	-30.363	1	0	1	-.05	1	-.426	1
503		5	max	3	26.736	3	.031	10	0	3	-.002	15	.178	3
504		min	-47.255	1	-64.468	1	-5.82	1	0	1	-.063	1	-.418	1
505		6	max	3	72.139	3	18.722	1	0	3	-.002	15	.144	3
506		min	-47.255	1	-172.034	1	.005	3	0	1	-.059	1	-.336	1
507		7	max	3	117.543	3	43.264	1	0	3	-.001	15	.078	3
508		min	-47.255	1	-279.599	1	.827	12	0	1	-.037	1	-.18	1
509		8	max	3	162.946	3	67.806	1	0	3	.002	2	.052	1
510		min	-47.255	1	-387.165	1	1.615	12	0	1	-.003	3	-.02	3
511		9	max	3	208.35	3	92.348	1	0	3	.057	1	.358	1
512		min	-47.255	1	-494.73	1	2.403	12	0	1	0	3	-.149	3
513		10	max	15	-12.492	15	116.89	1	0	15	.13	1	.739	1
514		min	-47.367	1	-602.296	1	-5.113	3	0	1	.003	12	-.309	3
515		11	max	15	494.73	1	-2.64	12	0	1	.057	1	.358	1
516		min	-47.367	1	-208.35	3	-91.998	1	0	3	.001	12	-.149	3
517		12	max	15	387.165	1	-1.852	12	0	1	.002	2	.052	1
518		min	-47.367	1	-162.946	3	-67.456	1	0	3	0	3	-.02	3
519		13	max	15	279.599	1	-1.065	12	0	1	-.001	15	.078	3
520		min	-47.367	1	-117.543	3	-42.913	1	0	3	-.037	1	-.18	1
521		14	max	15	172.033	1	-.277	12	0	1	-.002	12	.144	3
522		min	-47.367	1	-72.139	3	-18.371	1	0	3	-.058	1	-.336	1
523		15	max	15	64.468	1	6.171	1	0	1	-.002	12	.178	3
524		min	-47.367	1	-26.736	3	-.031	10	0	3	-.063	1	-.418	1
525		16	max	15	18.668	3	30.713	1	0	1	-.001	12	.181	3
526		min	-47.367	1	-43.098	1	1.031	15	0	3	-.05	1	-.426	1
527		17	max	15	64.071	3	55.255	1	0	1	0	3	.152	3
528		min	-47.367	1	-150.663	1	1.843	15	0	3	-.02	1	-.359	1
529		18	max	15	109.475	3	79.797	1	0	1	.027	1	.092	3
530		min	-47.367	1	-258.229	1	2.655	15	0	3	0	15	-.217	1
531		19	max	15	154.879	3	104.339	1	0	1	.091	1	0	1
532		min	-47.367	1	-365.795	1	3.467	15	0	3	.003	15	0	3
533	M15	1	max	2	2.183	4	.051	3	0	1	0	1	0	1
534		min	-44.512	3	0	2	-.054	1	0	3	0	3	0	1
535		2	max	2	1.941	4	.051	3	0	1	0	1	0	2
536		min	-44.572	3	0	2	-.054	1	0	3	0	3	0	4
537		3	max	2	1.698	4	.051	3	0	1	0	1	0	2
538		min	-44.631	3	0	2	-.054	1	0	3	0	3	-.001	4
539		4	max	2	1.456	4	.051	3	0	1	0	1	0	2
540		min	-44.691	3	0	2	-.054	1	0	3	0	3	-.002	4
541		5	max	2	1.213	4	.051	3	0	1	0	1	0	2
542		min	-44.751	3	0	2	-.054	1	0	3	0	3	-.003	4
543		6	max	2	.97	4	.051	3	0	1	0	1	0	2
544		min	-44.81	3	0	2	-.054	1	0	3	0	3	-.003	4
545		7	max	2	.728	4	.051	3	0	1	0	3	0	2
546		min	-44.87	3	0	2	-.054	1	0	3	0	1	-.003	4
547		8	max	2	.485	4	.051	3	0	1	0	3	0	2
548		min	-44.93	3	0	2	-.054	1	0	3	0	1	-.004	4
549		9	max	2	.243	4	.051	3	0	1	0	3	0	2
550		min	-44.989	3	0	2	-.054	1	0	3	0	1	-.004	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551		10	max	0	2	0	1	.051	3	0	1	0	3	0	2
552			min	-45.049	3	0	1	-.054	1	0	3	0	1	-.004	4
553		11	max	0	2	0	2	.051	3	0	1	0	3	0	2
554			min	-45.109	3	-.243	4	-.054	1	0	3	0	1	-.004	4
555		12	max	0	2	0	2	.051	3	0	1	0	3	0	2
556			min	-45.168	3	-.485	4	-.054	1	0	3	0	1	-.004	4
557		13	max	0	2	0	2	.051	3	0	1	0	3	0	2
558			min	-45.228	3	-.728	4	-.054	1	0	3	0	1	-.003	4
559		14	max	0	2	0	2	.051	3	0	1	0	3	0	2
560			min	-45.288	3	-.97	4	-.054	1	0	3	0	1	-.003	4
561		15	max	0	2	0	2	.051	3	0	1	0	3	0	2
562			min	-45.347	3	-1.213	4	-.054	1	0	3	0	1	-.003	4
563		16	max	0	2	0	2	.051	3	0	1	0	3	0	2
564			min	-45.407	3	-1.456	4	-.054	1	0	3	0	1	-.002	4
565		17	max	0	2	0	2	.051	3	0	1	0	3	0	2
566			min	-45.467	3	-1.698	4	-.054	1	0	3	0	1	-.001	4
567		18	max	0	2	0	2	.051	3	0	1	0	3	0	2
568			min	-45.526	3	-1.941	4	-.054	1	0	3	0	1	0	4
569		19	max	0	2	0	2	.051	3	0	1	0	3	0	1
570			min	-45.586	3	-2.183	4	-.054	1	0	3	0	1	0	1
571	M16A	1	max	0	10	2.183	4	.027	1	0	3	0	3	0	1
572			min	-44.865	3	0	10	-.02	3	0	1	0	1	0	1
573		2	max	0	10	1.941	4	.027	1	0	3	0	3	0	10
574			min	-44.806	3	0	10	-.02	3	0	1	0	1	0	4
575		3	max	0	10	1.698	4	.027	1	0	3	0	3	0	10
576			min	-44.746	3	0	10	-.02	3	0	1	0	1	-.001	4
577		4	max	0	10	1.456	4	.027	1	0	3	0	3	0	10
578			min	-44.686	3	0	10	-.02	3	0	1	0	1	-.002	4
579		5	max	0	10	1.213	4	.027	1	0	3	0	3	0	10
580			min	-44.627	3	0	10	-.02	3	0	1	0	1	-.003	4
581		6	max	0	10	.97	4	.027	1	0	3	0	3	0	10
582			min	-44.567	3	0	10	-.02	3	0	1	0	1	-.003	4
583		7	max	0	10	.728	4	.027	1	0	3	0	3	0	10
584			min	-44.507	3	0	10	-.02	3	0	1	0	1	-.003	4
585		8	max	0	10	.485	4	.027	1	0	3	0	3	0	10
586			min	-44.448	3	0	10	-.02	3	0	1	0	1	-.004	4
587		9	max	0	10	.243	4	.027	1	0	3	0	3	0	10
588			min	-44.388	3	0	10	-.02	3	0	1	0	1	-.004	4
589		10	max	0	10	0	1	.027	1	0	3	0	3	0	10
590			min	-44.328	3	0	1	-.02	3	0	1	0	1	-.004	4
591		11	max	0	10	0	10	.027	1	0	3	0	3	0	10
592			min	-44.269	3	-.243	4	-.02	3	0	1	0	1	-.004	4
593		12	max	0	10	0	10	.027	1	0	3	0	3	0	10
594			min	-44.209	3	-.485	4	-.02	3	0	1	0	1	-.004	4
595		13	max	0	10	0	10	.027	1	0	3	0	2	0	10
596			min	-44.149	3	-.728	4	-.02	3	0	1	0	4	-.003	4
597		14	max	0	10	0	10	.027	1	0	3	0	1	0	10
598			min	-44.09	3	-.97	4	-.02	3	0	1	0	3	-.003	4
599		15	max	0	10	0	10	.027	1	0	3	0	1	0	10
600			min	-44.03	3	-1.213	4	-.02	3	0	1	0	3	-.003	4
601		16	max	0	10	0	10	.027	1	0	3	0	1	0	10
602			min	-43.97	3	-1.456	4	-.02	3	0	1	0	3	-.002	4
603		17	max	0	10	0	10	.027	1	0	3	0	1	0	10
604			min	-43.911	3	-1.698	4	-.02	3	0	1	0	3	-.001	4
605		18	max	.015	2	0	10	.027	1	0	3	0	1	0	10
606			min	-43.851	3	-1.941	4	-.02	3	0	1	0	3	0	4
607		19	max	.095	2	0	10	.027	1	0	3	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-43.791	3	-2.183	4	-0.02	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.007	2	.009	1	-2.287e-5	15	NC	3	NC	2	
2			min	-.003	3	-.006	3	0	3	-6.723e-4	1	4957.802	2	3553.315	1	
3			2	max	.002	1	.006	2	.009	1	-2.194e-5	15	NC	3	NC	2
4				min	-.003	3	-.006	3	0	3	-6.453e-4	1	5391.368	2	3846.571	1
5			3	max	.002	1	.006	2	.008	1	-2.101e-5	15	NC	3	NC	2
6				min	-.003	3	-.006	3	0	3	-6.182e-4	1	5903.705	2	4192.164	1
7			4	max	.002	1	.005	2	.007	1	-2.008e-5	15	NC	1	NC	2
8				min	-.002	3	-.005	3	0	3	-5.912e-4	1	6513.401	2	4602.853	1
9			5	max	.002	1	.005	2	.007	1	-1.914e-5	15	NC	1	NC	2
10				min	-.002	3	-.005	3	0	3	-5.642e-4	1	7245.111	2	5095.681	1
11		6	max	.002	1	.004	2	.006	1	-1.821e-5	15	NC	1	NC	2	
12			min	-.002	3	-.005	3	0	3	-5.371e-4	1	8132.083	2	5693.841	1	
13		7	max	.002	1	.004	2	.005	1	-1.728e-5	15	NC	1	NC	2	
14			min	-.002	3	-.005	3	0	3	-5.101e-4	1	9220.011	2	6429.558	1	
15		8	max	.002	1	.003	2	.005	1	-1.635e-5	15	NC	1	NC	2	
16			min	-.002	3	-.004	3	0	3	-4.831e-4	1	NC	1	7348.7	1	
17		9	max	.001	1	.003	2	.004	1	-1.541e-5	15	NC	1	NC	2	
18			min	-.002	3	-.004	3	0	3	-4.56e-4	1	NC	1	8518.419	1	
19		10	max	.001	1	.002	2	.003	1	-1.448e-5	15	NC	1	NC	1	
20			min	-.001	3	-.004	3	0	3	-4.29e-4	1	NC	1	NC	1	
21		11	max	.001	1	.002	2	.003	1	-1.355e-5	15	NC	1	NC	1	
22			min	-.001	3	-.003	3	0	3	-4.02e-4	1	NC	1	NC	1	
23		12	max	.001	1	.002	2	.002	1	-1.262e-5	15	NC	1	NC	1	
24			min	-.001	3	-.003	3	0	3	-3.749e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	.002	1	-1.168e-5	15	NC	1	NC	1	
26			min	0	3	-.003	3	0	3	-3.479e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.001	1	-1.075e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-3.209e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-9.817e-6	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-2.938e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-8.885e-6	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-2.668e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-7.952e-6	15	NC	1	NC	1	
34			min	0	3	0	3	0	3	-2.398e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-7.02e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	3	-2.127e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-5.751e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.857e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	8.526e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	2.778e-6	12	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	1.065e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	3.523e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	1.278e-4	1	NC	1	NC	1
44				min	0	2	-.001	3	0	1	4.249e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	12	1.491e-4	1	NC	1	NC	1
46				min	0	2	-.002	3	0	1	4.976e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	1.704e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	0	1	5.703e-6	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	1.917e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	0	1	6.429e-6	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	2.13e-4	1	NC	1	NC	1	



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.004	3	0	1	7.156e-6	15	NC	1	NC	1
53		8	max	0	3	0	2	0	3	2.343e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	1	7.882e-6	15	NC	1	NC	1
55		9	max	0	3	.001	2	0	3	2.556e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	1	8.609e-6	15	NC	1	NC	1
57		10	max	0	3	.001	2	0	1	2.769e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	15	9.336e-6	15	NC	1	NC	1
59		11	max	0	3	.002	2	0	1	2.982e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	15	1.006e-5	15	NC	1	NC	1
61		12	max	0	3	.002	2	.001	1	3.195e-4	1	NC	1	NC	1
62			min	0	2	-.006	3	0	15	1.079e-5	15	NC	1	NC	1
63		13	max	0	3	.003	2	.002	1	3.408e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	15	1.152e-5	15	NC	1	NC	1
65		14	max	0	3	.004	2	.002	1	3.62e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	15	1.224e-5	15	NC	1	NC	1
67		15	max	0	3	.005	2	.003	1	3.833e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	15	1.297e-5	15	9851.229	2	NC	1
69		16	max	0	3	.006	2	.003	1	4.046e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	15	1.37e-5	15	8295.427	2	NC	1
71		17	max	0	3	.006	2	.003	1	4.259e-4	1	NC	3	NC	1
72			min	0	2	-.007	3	0	15	1.442e-5	15	7102.038	2	NC	1
73		18	max	0	3	.007	2	.004	1	4.472e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	15	1.515e-5	15	6175.267	2	NC	1
75		19	max	0	3	.008	2	.004	1	4.685e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	15	1.588e-5	15	5448.539	2	NC	1
77	M4	1	max	.002	1	.008	2	0	15	-1.926e-5	15	NC	1	NC	2
78			min	0	3	-.006	3	-.003	1	-5.786e-4	1	NC	1	5881.548	1
79		2	max	.002	1	.007	2	0	15	-1.926e-5	15	NC	1	NC	2
80			min	0	3	-.006	3	-.003	1	-5.786e-4	1	NC	1	6417.642	1
81		3	max	.002	1	.007	2	0	15	-1.926e-5	15	NC	1	NC	2
82			min	0	3	-.006	3	-.003	1	-5.786e-4	1	NC	1	7055.613	1
83		4	max	.002	1	.006	2	0	15	-1.926e-5	15	NC	1	NC	2
84			min	0	3	-.005	3	-.002	1	-5.786e-4	1	NC	1	7822.347	1
85		5	max	.002	1	.006	2	0	15	-1.926e-5	15	NC	1	NC	2
86			min	0	3	-.005	3	-.002	1	-5.786e-4	1	NC	1	8754.469	1
87		6	max	.002	1	.006	2	0	15	-1.926e-5	15	NC	1	NC	2
88			min	0	3	-.004	3	-.002	1	-5.786e-4	1	NC	1	9902.921	1
89		7	max	.001	1	.005	2	0	15	-1.926e-5	15	NC	1	NC	1
90			min	0	3	-.004	3	-.002	1	-5.786e-4	1	NC	1	NC	1
91		8	max	.001	1	.005	2	0	15	-1.926e-5	15	NC	1	NC	1
92			min	0	3	-.004	3	-.001	1	-5.786e-4	1	NC	1	NC	1
93		9	max	.001	1	.004	2	0	15	-1.926e-5	15	NC	1	NC	1
94			min	0	3	-.003	3	-.001	1	-5.786e-4	1	NC	1	NC	1
95		10	max	.001	1	.004	2	0	15	-1.926e-5	15	NC	1	NC	1
96			min	0	3	-.003	3	-.001	1	-5.786e-4	1	NC	1	NC	1
97		11	max	0	1	.003	2	0	15	-1.926e-5	15	NC	1	NC	1
98			min	0	3	-.003	3	0	1	-5.786e-4	1	NC	1	NC	1
99		12	max	0	1	.003	2	0	15	-1.926e-5	15	NC	1	NC	1
100			min	0	3	-.002	3	0	1	-5.786e-4	1	NC	1	NC	1
101		13	max	0	1	.003	2	0	15	-1.926e-5	15	NC	1	NC	1
102			min	0	3	-.002	3	0	1	-5.786e-4	1	NC	1	NC	1
103		14	max	0	1	.002	2	0	15	-1.926e-5	15	NC	1	NC	1
104			min	0	3	-.002	3	0	1	-5.786e-4	1	NC	1	NC	1
105		15	max	0	1	.002	2	0	15	-1.926e-5	15	NC	1	NC	1
106			min	0	3	-.001	3	0	1	-5.786e-4	1	NC	1	NC	1
107		16	max	0	1	.001	2	0	15	-1.926e-5	15	NC	1	NC	1
108			min	0	3	-.001	3	0	1	-5.786e-4	1	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-1.926e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-5.786e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.926e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-5.786e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.926e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-5.786e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.024	2	.004	1	2.435e-4	3	NC	3	NC	2
116			min	-.01	3	-.019	3	-.003	3	-6.118e-8	10	1382.269	2	8751.789	1
117		2	max	.008	1	.023	2	.004	1	2.371e-4	3	NC	3	NC	2
118			min	-.009	3	-.018	3	-.003	3	-5.787e-8	10	1475.854	2	9446.255	1
119		3	max	.007	1	.021	2	.003	1	2.306e-4	3	NC	3	NC	1
120			min	-.009	3	-.017	3	-.003	3	-5.457e-8	10	1582.69	2	NC	1
121		4	max	.007	1	.019	2	.003	1	2.242e-4	3	NC	3	NC	1
122			min	-.008	3	-.016	3	-.003	3	-5.126e-8	10	1705.426	2	NC	1
123		5	max	.007	1	.018	2	.003	1	2.177e-4	3	NC	3	NC	1
124			min	-.007	3	-.015	3	-.002	3	-4.137e-7	2	1847.476	2	NC	1
125		6	max	.006	1	.017	2	.002	1	2.112e-4	3	NC	3	NC	1
126			min	-.007	3	-.014	3	-.002	3	-2.485e-6	2	2013.309	2	NC	1
127		7	max	.006	1	.015	2	.002	1	2.048e-4	3	NC	3	NC	1
128			min	-.006	3	-.013	3	-.002	3	-4.556e-6	2	2208.885	2	NC	1
129		8	max	.005	1	.014	2	.002	1	1.983e-4	3	NC	3	NC	1
130			min	-.006	3	-.012	3	-.002	3	-6.627e-6	2	2442.338	2	NC	1
131		9	max	.005	1	.012	2	.002	1	1.919e-4	3	NC	3	NC	1
132			min	-.005	3	-.011	3	-.001	3	-8.698e-6	2	2725.06	2	NC	1
133		10	max	.004	1	.011	2	.001	1	1.854e-4	3	NC	3	NC	1
134			min	-.005	3	-.01	3	-.001	3	-1.172e-5	11	3073.506	2	NC	1
135		11	max	.004	1	.009	2	.001	1	1.79e-4	3	NC	3	NC	1
136			min	-.004	3	-.009	3	-.001	3	-1.518e-5	1	3512.364	2	NC	1
137		12	max	.003	1	.008	2	0	1	1.725e-4	3	NC	3	NC	1
138			min	-.004	3	-.008	3	0	3	-2.211e-5	1	4080.426	2	NC	1
139		13	max	.003	1	.007	2	0	1	1.661e-4	3	NC	3	NC	1
140			min	-.003	3	-.007	3	0	3	-2.905e-5	1	4842.348	2	NC	1
141		14	max	.002	1	.006	2	0	1	1.596e-4	3	NC	3	NC	1
142			min	-.003	3	-.006	3	0	3	-3.598e-5	1	5914.504	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.532e-4	3	NC	3	NC	1
144			min	-.002	3	-.005	3	0	3	-4.292e-5	1	7529.629	2	NC	1
145		16	max	.001	1	.003	2	0	1	1.467e-4	3	NC	1	NC	1
146			min	-.002	3	-.003	3	0	3	-4.985e-5	1	NC	1	NC	1
147		17	max	0	1	.002	2	0	1	1.402e-4	3	NC	1	NC	1
148			min	-.001	3	-.002	3	0	3	-5.679e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.338e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-6.372e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.273e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-7.066e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.204e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-5.825e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	2.708e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-4.403e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.213e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-2.98e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.717e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.558e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	.001	3	1.221e-5	1	NC	1	NC	1
162			min	0	2	-.007	3	0	1	-1.354e-6	3	8719.243	2	NC	1
163		6	max	0	3	.007	2	.001	3	1.287e-5	3	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	0	10	6987.727	2	NC	1
165		7	max	0	3	.008	2	.001	3	2.71e-5	3	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.001	2	-.01	3	0	1	0	10	5803.399	2	NC	1
167		8	max	.001	3	.009	2	.002	3	4.132e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	-.001	1	-2.662e-6	1	4935.04	2	NC	1
169		9	max	.001	3	.011	2	.002	3	5.555e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	-.001	1	-7.619e-6	1	4267.451	2	NC	1
171		10	max	.001	3	.012	2	.002	3	6.977e-5	3	NC	3	NC	1
172			min	-.002	2	-.014	3	-.001	1	-1.258e-5	1	3736.779	2	NC	1
173		11	max	.002	3	.014	2	.002	3	8.399e-5	3	NC	3	NC	1
174			min	-.002	2	-.015	3	-.001	1	-1.753e-5	1	3304.675	2	NC	1
175		12	max	.002	3	.016	2	.002	3	9.822e-5	3	NC	3	NC	1
176			min	-.002	2	-.016	3	-.002	1	-2.249e-5	1	2946.588	2	NC	1
177		13	max	.002	3	.017	2	.002	3	1.124e-4	3	NC	3	NC	1
178			min	-.003	2	-.017	3	-.002	1	-2.745e-5	1	2645.948	2	NC	1
179		14	max	.002	3	.019	2	.002	3	1.267e-4	3	NC	3	NC	1
180			min	-.003	2	-.018	3	-.002	1	-3.241e-5	1	2391.069	2	NC	1
181		15	max	.002	3	.021	2	.002	3	1.409e-4	3	NC	3	NC	1
182			min	-.003	2	-.019	3	-.002	1	-3.737e-5	1	2173.395	2	NC	1
183		16	max	.002	3	.023	2	.002	3	1.551e-4	3	NC	3	NC	1
184			min	-.003	2	-.02	3	-.002	1	-4.232e-5	1	1986.471	2	NC	1
185		17	max	.003	3	.025	2	.002	3	1.693e-4	3	NC	3	NC	1
186			min	-.003	2	-.021	3	-.002	1	-4.728e-5	1	1825.304	2	NC	1
187		18	max	.003	3	.027	2	.002	3	1.836e-4	3	NC	3	NC	1
188			min	-.004	2	-.022	3	-.002	1	-5.224e-5	1	1685.958	2	NC	1
189		19	max	.003	3	.029	2	.002	3	1.978e-4	3	NC	3	NC	1
190			min	-.004	2	-.022	3	-.002	1	-5.72e-5	1	1565.285	2	NC	1
191	M8	1	max	.006	1	.027	2	.002	1	-7.356e-8	10	NC	1	NC	2
192			min	-.001	3	-.02	3	-.001	3	-1.543e-4	3	NC	1	9990.108	1
193		2	max	.006	1	.026	2	.002	1	-7.356e-8	10	NC	1	NC	1
194			min	-.001	3	-.019	3	-.001	3	-1.543e-4	3	NC	1	NC	1
195		3	max	.005	1	.024	2	.002	1	-7.356e-8	10	NC	1	NC	1
196			min	-.001	3	-.018	3	0	3	-1.543e-4	3	NC	1	NC	1
197		4	max	.005	1	.023	2	.001	1	-7.356e-8	10	NC	1	NC	1
198			min	-.001	3	-.016	3	0	3	-1.543e-4	3	NC	1	NC	1
199		5	max	.005	1	.021	2	.001	1	-7.356e-8	10	NC	1	NC	1
200			min	0	3	-.015	3	0	3	-1.543e-4	3	NC	1	NC	1
201		6	max	.004	1	.02	2	.001	1	-7.356e-8	10	NC	1	NC	1
202			min	0	3	-.014	3	0	3	-1.543e-4	3	NC	1	NC	1
203		7	max	.004	1	.018	2	.001	1	-7.356e-8	10	NC	1	NC	1
204			min	0	3	-.013	3	0	3	-1.543e-4	3	NC	1	NC	1
205		8	max	.004	1	.017	2	0	1	-7.356e-8	10	NC	1	NC	1
206			min	0	3	-.012	3	0	3	-1.543e-4	3	NC	1	NC	1
207		9	max	.003	1	.015	2	0	1	-7.356e-8	10	NC	1	NC	1
208			min	0	3	-.011	3	0	3	-1.543e-4	3	NC	1	NC	1
209		10	max	.003	1	.014	2	0	1	-7.356e-8	10	NC	1	NC	1
210			min	0	3	-.01	3	0	3	-1.543e-4	3	NC	1	NC	1
211		11	max	.003	1	.012	2	0	1	-7.356e-8	10	NC	1	NC	1
212			min	0	3	-.009	3	0	3	-1.543e-4	3	NC	1	NC	1
213		12	max	.002	1	.011	2	0	1	-7.356e-8	10	NC	1	NC	1
214			min	0	3	-.008	3	0	3	-1.543e-4	3	NC	1	NC	1
215		13	max	.002	1	.009	2	0	1	-7.356e-8	10	NC	1	NC	1
216			min	0	3	-.007	3	0	3	-1.543e-4	3	NC	1	NC	1
217		14	max	.002	1	.008	2	0	1	-7.356e-8	10	NC	1	NC	1
218			min	0	3	-.005	3	0	3	-1.543e-4	3	NC	1	NC	1
219		15	max	.001	1	.006	2	0	1	-7.356e-8	10	NC	1	NC	1
220			min	0	3	-.004	3	0	3	-1.543e-4	3	NC	1	NC	1
221		16	max	.001	1	.005	2	0	1	-7.356e-8	10	NC	1	NC	1
222			min	0	3	-.003	3	0	3	-1.543e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.004	3	-.001	3	-2.541e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	10	-9.696e-6	15	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-2.893e-4	1	NC	1	NC	1
283		9	max	0	3	.001	2	0	10	-1.094e-5	15	NC	1	NC	1
284			min	0	2	-.005	3	-.002	1	-3.245e-4	1	NC	1	NC	1
285		10	max	0	3	.001	2	0	15	-1.219e-5	15	NC	1	NC	1
286			min	0	2	-.006	3	-.003	1	-3.597e-4	1	NC	1	NC	1
287		11	max	0	3	.002	2	0	15	-1.343e-5	15	NC	1	NC	1
288			min	0	2	-.006	3	-.003	1	-3.949e-4	1	NC	1	NC	1
289		12	max	0	3	.002	2	0	15	-1.468e-5	15	NC	1	NC	1
290			min	0	2	-.007	3	-.004	1	-4.301e-4	1	NC	1	NC	1
291		13	max	0	3	.003	2	0	15	-1.593e-5	15	NC	1	NC	2
292			min	0	2	-.007	3	-.005	1	-4.653e-4	1	NC	1	9697.117	1
293		14	max	0	3	.004	2	0	15	-1.717e-5	15	NC	1	NC	2
294			min	0	2	-.007	3	-.005	1	-5.005e-4	1	NC	1	8402.069	1
295		15	max	0	3	.005	2	0	15	-1.842e-5	15	NC	1	NC	2
296			min	0	2	-.007	3	-.006	1	-5.357e-4	1	9867.28	2	7414.971	1
297		16	max	0	3	.006	2	0	15	-1.967e-5	15	NC	1	NC	2
298			min	0	2	-.007	3	-.007	1	-5.708e-4	1	8307.489	2	6648.768	1
299		17	max	0	3	.006	2	0	15	-2.091e-5	15	NC	3	NC	2
300			min	0	2	-.007	3	-.008	1	-6.06e-4	1	7111.374	2	6046.271	1
301		18	max	0	3	.007	2	0	15	-2.216e-5	15	NC	3	NC	2
302			min	0	2	-.007	3	-.008	1	-6.412e-4	1	6182.697	2	5568.769	1
303		19	max	0	3	.008	2	0	15	-2.34e-5	15	NC	3	NC	2
304			min	0	2	-.007	3	-.009	1	-6.764e-4	1	5454.611	2	5189.486	1
305	M12	1	max	.002	1	.008	2	.007	1	5.706e-4	1	NC	1	NC	3
306			min	0	3	-.006	3	0	15	2.026e-5	15	NC	1	2591.551	1
307		2	max	.002	1	.007	2	.007	1	5.706e-4	1	NC	1	NC	3
308			min	0	3	-.006	3	0	15	2.026e-5	15	NC	1	2826.517	1
309		3	max	.002	1	.007	2	.006	1	5.706e-4	1	NC	1	NC	3
310			min	0	3	-.006	3	0	15	2.026e-5	15	NC	1	3106.194	1
311		4	max	.002	1	.006	2	.006	1	5.706e-4	1	NC	1	NC	2
312			min	0	3	-.005	3	0	15	2.026e-5	15	NC	1	3442.37	1
313		5	max	.002	1	.006	2	.005	1	5.706e-4	1	NC	1	NC	2
314			min	0	3	-.005	3	0	15	2.026e-5	15	NC	1	3851.103	1
315		6	max	.002	1	.006	2	.004	1	5.706e-4	1	NC	1	NC	2
316			min	0	3	-.004	3	0	15	2.026e-5	15	NC	1	4354.727	1
317		7	max	.001	1	.005	2	.004	1	5.706e-4	1	NC	1	NC	2
318			min	0	3	-.004	3	0	15	2.026e-5	15	NC	1	4985.058	1
319		8	max	.001	1	.005	2	.003	1	5.706e-4	1	NC	1	NC	2
320			min	0	3	-.004	3	0	15	2.026e-5	15	NC	1	5788.685	1
321		9	max	.001	1	.004	2	.003	1	5.706e-4	1	NC	1	NC	2
322			min	0	3	-.003	3	0	15	2.026e-5	15	NC	1	6836.083	1
323		10	max	.001	1	.004	2	.002	1	5.706e-4	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	2.026e-5	15	NC	1	8238.052	1
325		11	max	0	1	.003	2	.002	1	5.706e-4	1	NC	1	NC	1
326			min	0	3	-.003	3	0	15	2.026e-5	15	NC	1	NC	1
327		12	max	0	1	.003	2	.001	1	5.706e-4	1	NC	1	NC	1
328			min	0	3	-.002	3	0	15	2.026e-5	15	NC	1	NC	1
329		13	max	0	1	.003	2	.001	1	5.706e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	2.026e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	0	1	5.706e-4	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	2.026e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	5.706e-4	1	NC	1	NC	1
334			min	0	3	-.001	3	0	15	2.026e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	5.706e-4	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	2.026e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	5.706e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	2.026e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	5.706e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	2.026e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	5.706e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	2.026e-5	15	NC	1	NC	1
343	M1	1	max	.006	3	.022	3	.002	3	1.719e-2	1	NC	1	NC	1
344			min	-.007	2	-.023	1	-.004	1	-1.83e-2	3	NC	1	NC	1
345		2	max	.006	3	.012	3	.001	3	8.301e-3	1	NC	4	NC	1
346			min	-.007	2	-.012	1	-.007	1	-9.047e-3	3	4507.234	1	NC	1
347		3	max	.006	3	.003	3	0	3	3.355e-5	3	NC	4	NC	2
348			min	-.007	2	-.003	1	-.009	1	-4.209e-4	1	2325.107	1	8042.894	1
349		4	max	.006	3	.006	1	0	3	3.475e-5	3	NC	4	NC	2
350			min	-.007	2	-.004	3	-.011	1	-3.541e-4	1	1642.947	1	6665.189	1
351		5	max	.006	3	.013	1	0	3	3.594e-5	3	NC	5	NC	2
352			min	-.007	2	-.01	3	-.011	1	-2.874e-4	1	1315.087	1	6413.207	1
353		6	max	.006	3	.018	1	0	3	3.714e-5	3	NC	5	NC	2
354			min	-.007	2	-.015	3	-.01	1	-2.206e-4	1	1129.692	1	6882.937	1
355		7	max	.006	3	.023	1	0	3	3.833e-5	3	NC	5	NC	2
356			min	-.007	2	-.019	3	-.009	1	-1.539e-4	1	1017.326	1	8235.894	1
357		8	max	.006	3	.026	1	0	3	3.953e-5	3	NC	5	NC	1
358			min	-.007	2	-.021	3	-.007	1	-8.711e-5	1	949.116	1	NC	1
359		9	max	.006	3	.028	1	0	3	4.072e-5	3	NC	5	NC	1
360			min	-.007	2	-.023	3	-.005	1	-2.036e-5	1	911.575	1	NC	1
361		10	max	.006	3	.028	1	0	3	4.64e-5	1	NC	5	NC	1
362			min	-.007	2	-.023	3	-.003	1	1.869e-6	15	898.467	1	NC	1
363		11	max	.006	3	.028	1	0	3	1.132e-4	1	NC	5	NC	1
364			min	-.007	2	-.022	3	0	1	4.089e-6	15	907.869	1	NC	1
365		12	max	.006	3	.026	2	0	1	1.799e-4	1	NC	5	NC	1
366			min	-.007	2	-.02	3	0	15	6.31e-6	15	940.881	2	NC	1
367		13	max	.006	3	.023	2	.003	1	2.467e-4	1	NC	5	NC	2
368			min	-.007	2	-.018	3	0	15	8.531e-6	15	998.151	2	8652.234	1
369		14	max	.006	3	.018	2	.004	1	3.134e-4	1	NC	5	NC	2
370			min	-.007	2	-.014	3	0	15	1.075e-5	15	1097.446	2	7140.234	1
371		15	max	.006	3	.012	2	.005	1	3.802e-4	1	NC	5	NC	2
372			min	-.007	2	-.009	3	0	15	1.297e-5	15	1265.676	2	6601.811	1
373		16	max	.006	3	.005	2	.004	1	4.289e-4	1	NC	4	NC	2
374			min	-.007	2	-.004	3	0	15	1.46e-5	15	1568.07	2	6821.288	1
375		17	max	.006	3	.002	3	.003	1	4.886e-5	1	NC	4	NC	2
376			min	-.007	2	-.004	2	0	15	2.158e-6	15	2208.526	2	8198.64	1
377		18	max	.006	3	.009	3	.001	1	9.861e-3	1	NC	4	NC	1
378			min	-.007	2	-.015	2	0	15	-4.246e-3	3	4270.442	2	NC	1
379		19	max	.006	3	.016	3	0	3	1.987e-2	1	NC	1	NC	1
380			min	-.007	2	-.026	2	-.002	1	-8.605e-3	3	NC	1	NC	1
381	M5	1	max	.018	3	.071	3	.002	3	1.235e-6	3	NC	1	NC	1
382			min	-.024	2	-.076	1	-.004	1	3.484e-8	15	NC	1	NC	1
383		2	max	.018	3	.039	3	.002	3	6.409e-5	3	NC	5	NC	1
384			min	-.024	2	-.041	1	-.004	1	-7.396e-5	1	1331.205	1	NC	1
385		3	max	.018	3	.01	3	.003	3	1.258e-4	3	NC	5	NC	1
386			min	-.024	2	-.009	1	-.004	1	-1.466e-4	1	686.031	1	NC	1
387		4	max	.018	3	.019	1	.004	3	1.238e-4	3	NC	5	NC	1
388			min	-.024	2	-.014	3	-.004	1	-1.393e-4	1	483.851	1	NC	1
389		5	max	.018	3	.043	1	.004	3	1.218e-4	3	NC	5	NC	1
390			min	-.024	2	-.034	3	-.003	1	-1.32e-4	1	386.551	1	NC	1
391		6	max	.018	3	.063	1	.004	3	1.199e-4	3	NC	5	NC	1
392			min	-.024	2	-.05	3	-.003	1	-1.247e-4	1	331.426	1	NC	1
393		7	max	.018	3	.078	1	.004	3	1.179e-4	3	NC	15	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.024	2	-.062	3	-.003	1	-1.174e-4	1	297.911	1	NC	1
395		8	max	.018	3	.089	1	.004	3	1.16e-4	3	NC	15	NC	1
396			min	-.024	2	-.07	3	-.003	1	-1.101e-4	1	277.445	1	NC	1
397		9	max	.018	3	.096	1	.004	3	1.14e-4	3	NC	15	NC	1
398			min	-.024	2	-.074	3	-.003	1	-1.028e-4	1	266.023	1	NC	1
399		10	max	.018	3	.098	1	.004	3	1.121e-4	3	NC	15	NC	1
400			min	-.024	2	-.075	3	-.003	1	-9.554e-5	1	261.784	1	NC	1
401		11	max	.018	3	.096	1	.004	3	1.101e-4	3	NC	15	NC	1
402			min	-.024	2	-.072	3	-.003	1	-8.825e-5	1	264.138	1	NC	1
403		12	max	.018	3	.089	1	.003	3	1.081e-4	3	NC	15	NC	1
404			min	-.024	2	-.066	3	-.002	1	-8.095e-5	1	273.531	1	NC	1
405		13	max	.018	3	.078	1	.003	3	1.062e-4	3	NC	15	NC	1
406			min	-.024	2	-.057	3	-.002	1	-7.366e-5	1	291.642	1	NC	1
407		14	max	.018	3	.062	1	.002	3	1.042e-4	3	NC	5	NC	1
408			min	-.024	2	-.045	3	-.002	1	-6.636e-5	1	322.194	1	NC	1
409		15	max	.018	3	.042	1	.002	3	1.023e-4	3	NC	5	NC	1
410			min	-.024	2	-.03	3	-.002	1	-5.906e-5	1	373.218	1	NC	1
411		16	max	.018	3	.017	1	.002	3	9.758e-5	3	NC	5	NC	1
412			min	-.024	2	-.013	3	-.002	1	-5.578e-5	1	464.039	1	NC	1
413		17	max	.018	3	.008	3	.001	3	2.785e-5	3	NC	5	NC	1
414			min	-.024	2	-.014	2	-.002	1	-1.479e-4	1	654.126	1	NC	1
415		18	max	.018	3	.03	3	0	3	1.33e-5	3	NC	5	NC	1
416			min	-.024	2	-.049	2	-.002	1	-7.553e-5	1	1265.725	1	NC	1
417		19	max	.018	3	.053	3	0	3	0	1	NC	1	NC	1
418			min	-.024	2	-.087	2	-.002	1	-2.102e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.021	3	.001	3	1.83e-2	3	NC	1	NC	1
420			min	-.007	2	-.023	1	-.005	1	-1.719e-2	1	NC	1	NC	1
421		2	max	.006	3	.012	3	0	3	9.068e-3	3	NC	4	NC	1
422			min	-.007	2	-.012	1	0	1	-8.479e-3	1	4508.518	1	NC	1
423		3	max	.006	3	.003	3	.001	1	6.889e-5	1	NC	4	NC	2
424			min	-.007	2	-.003	1	0	3	2.54e-6	15	2325.789	1	8499.025	1
425		4	max	.006	3	.006	1	.003	1	2.311e-5	2	NC	4	NC	2
426			min	-.007	2	-.004	3	0	3	-3.786e-6	3	1643.427	1	7080.685	1
427		5	max	.006	3	.013	1	.003	1	8.939e-6	10	NC	5	NC	2
428			min	-.007	2	-.011	3	-.001	3	-3.909e-5	1	1315.45	1	6858.181	1
429		6	max	.006	3	.018	1	.003	1	4.688e-6	10	NC	5	NC	2
430			min	-.007	2	-.015	3	-.002	3	-9.308e-5	1	1129.982	1	7430.954	1
431		7	max	.006	3	.023	1	.002	1	4.381e-7	10	NC	5	NC	2
432			min	-.007	2	-.019	3	-.002	3	-1.471e-4	1	1017.563	1	9031.936	1
433		8	max	.006	3	.026	1	0	2	-3.812e-6	10	NC	5	NC	1
434			min	-.007	2	-.022	3	-.003	3	-2.011e-4	1	949.314	1	NC	1
435		9	max	.006	3	.028	1	0	10	-8.062e-6	10	NC	5	NC	1
436			min	-.007	2	-.023	3	-.003	3	-2.551e-4	1	911.743	1	NC	1
437		10	max	.006	3	.028	1	0	10	-1.028e-5	15	NC	5	NC	1
438			min	-.007	2	-.023	3	-.003	1	-3.09e-4	1	898.61	1	NC	1
439		11	max	.006	3	.028	1	0	10	-1.212e-5	15	NC	5	NC	1
440			min	-.007	2	-.022	3	-.005	1	-3.63e-4	1	907.989	1	NC	1
441		12	max	.006	3	.026	2	0	15	-1.395e-5	15	NC	5	NC	1
442			min	-.007	2	-.02	3	-.007	1	-4.17e-4	1	941.334	2	NC	1
443		13	max	.006	3	.023	2	0	15	-1.578e-5	15	NC	5	NC	2
444			min	-.007	2	-.018	3	-.008	1	-4.71e-4	1	998.62	2	8390.898	1
445		14	max	.006	3	.018	2	0	15	-1.761e-5	15	NC	5	NC	2
446			min	-.007	2	-.014	3	-.009	1	-5.25e-4	1	1097.948	2	7029.195	1
447		15	max	.006	3	.012	2	0	15	-1.945e-5	15	NC	5	NC	2
448			min	-.007	2	-.009	3	-.009	1	-5.79e-4	1	1266.237	2	6556.657	1
449		16	max	.006	3	.005	2	0	15	-2.082e-5	15	NC	4	NC	2
450			min	-.007	2	-.004	3	-.009	1	-6.2e-4	1	1568.738	2	6812.241	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.006	3	.002	3	0	15	-5.491e-6	12	NC	4	NC	2
452			min	-.007	2	-.004	2	-.007	1	-3.509e-4	1	2209.402	2	8217.994	1
453		18	max	.006	3	.009	3	0	15	4.259e-3	3	NC	4	NC	1
454			min	-.007	2	-.015	2	-.005	1	-1.001e-2	1	4272.078	2	NC	1
455		19	max	.006	3	.016	3	0	3	8.604e-3	3	NC	1	NC	1
456			min	-.007	2	-.026	2	-.001	1	-1.987e-2	1	NC	1	NC	1
457	M13	1	max	.005	1	.021	3	.006	3	3.633e-3	3	NC	1	NC	1
458			min	-.001	3	-.023	1	-.007	2	-3.893e-3	1	NC	1	NC	1
459		2	max	.004	1	.169	3	.019	1	4.549e-3	3	NC	5	NC	2
460			min	-.001	3	-.161	1	-.002	10	-4.921e-3	1	1018.578	3	6480.253	1
461		3	max	.004	1	.289	3	.05	1	5.465e-3	3	NC	5	NC	3
462			min	-.001	3	-.274	1	0	10	-5.948e-3	1	559.983	3	2769.704	1
463		4	max	.004	1	.365	3	.076	1	6.38e-3	3	NC	5	NC	3
464			min	-.001	3	-.346	1	0	10	-6.976e-3	1	436.272	3	1881.851	1
465		5	max	.004	1	.388	3	.087	1	7.296e-3	3	NC	5	NC	3
466			min	-.002	3	-.369	1	0	10	-8.004e-3	1	409.131	3	1642.122	1
467		6	max	.004	1	.359	3	.081	1	8.212e-3	3	NC	5	NC	3
468			min	-.002	3	-.342	1	-.001	10	-9.031e-3	1	444.756	3	1754.867	1
469		7	max	.004	1	.288	3	.059	1	9.128e-3	3	NC	5	NC	3
470			min	-.002	3	-.277	1	-.004	10	-1.006e-2	1	563.555	3	2366.746	1
471		8	max	.004	1	.195	3	.027	1	1.004e-2	3	NC	5	NC	2
472			min	-.002	3	-.191	1	-.008	10	-1.109e-2	1	865.719	3	4809.033	1
473		9	max	.004	1	.11	3	.017	3	1.096e-2	3	NC	5	NC	1
474			min	-.002	3	-.112	1	-.018	2	-1.211e-2	1	1674.854	1	NC	1
475		10	max	.004	1	.071	3	.018	3	1.187e-2	3	NC	4	NC	1
476			min	-.002	3	-.076	1	-.024	2	-1.314e-2	1	2800.723	1	8804.989	2
477		11	max	.004	1	.11	3	.021	3	1.096e-2	3	NC	5	NC	1
478			min	-.002	3	-.112	1	-.017	2	-1.211e-2	1	1674.855	1	NC	1
479		12	max	.004	1	.195	3	.028	1	1.004e-2	3	NC	5	NC	2
480			min	-.002	3	-.191	1	-.008	10	-1.109e-2	1	865.719	3	4662.022	1
481		13	max	.004	1	.288	3	.06	1	9.129e-3	3	NC	5	NC	5
482			min	-.002	3	-.277	1	-.004	10	-1.006e-2	1	563.555	3	2329.436	1
483		14	max	.004	1	.359	3	.082	1	8.214e-3	3	NC	5	NC	5
484			min	-.002	3	-.342	1	-.001	10	-9.032e-3	1	444.756	3	1737.575	1
485		15	max	.004	1	.388	3	.088	1	7.299e-3	3	NC	5	NC	3
486			min	-.002	3	-.369	1	0	10	-8.004e-3	1	409.131	3	1631.878	1
487		16	max	.004	1	.365	3	.076	1	6.384e-3	3	NC	5	NC	3
488			min	-.002	3	-.346	1	0	10	-6.976e-3	1	436.272	3	1875.579	1
489		17	max	.004	1	.29	3	.05	1	5.469e-3	3	NC	5	NC	3
490			min	-.002	3	-.274	1	0	10	-5.949e-3	1	559.983	3	2769.026	1
491		18	max	.004	1	.169	3	.019	1	4.554e-3	3	NC	5	NC	2
492			min	-.002	3	-.161	1	-.002	10	-4.921e-3	1	1018.578	3	6509.308	1
493		19	max	.004	1	.022	3	.006	3	3.638e-3	3	NC	1	NC	1
494			min	-.002	3	-.023	1	-.007	2	-3.893e-3	1	NC	1	NC	1
495	M16	1	max	.001	1	.016	3	.006	3	4.265e-3	2	NC	1	NC	1
496			min	0	3	-.026	2	-.007	2	-2.743e-3	3	NC	1	NC	1
497		2	max	.001	1	.086	3	.019	1	5.378e-3	2	NC	5	NC	2
498			min	0	3	-.185	1	-.002	10	-3.423e-3	3	938.098	1	6409.902	1
499		3	max	.002	1	.143	3	.05	1	6.49e-3	2	NC	5	NC	3
500			min	0	3	-.316	1	0	10	-4.103e-3	3	515.372	1	2748.093	1
501		4	max	.002	1	.18	3	.076	1	7.603e-3	2	NC	5	NC	3
502			min	0	3	-.399	1	0	10	-4.782e-3	3	400.998	1	1869.678	1
503		5	max	.002	1	.192	3	.088	1	8.716e-3	2	NC	5	NC	3
504			min	0	3	-.425	1	0	10	-5.462e-3	3	375.232	1	1632.53	1
505		6	max	.002	1	.18	3	.082	1	9.829e-3	2	NC	5	NC	5
506			min	0	3	-.394	1	-.001	10	-6.142e-3	3	406.368	1	1745.015	1
507		7	max	.002	1	.15	3	.059	1	1.094e-2	2	NC	5	NC	5



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.318	1	-.004	10	-6.822e-3	3	511.253	1	2353.373	1
509	8	max	.002	1	.109	3	.027	1	1.206e-2	2	NC	5	NC	2
510		min	0	3	-.219	1	-.008	10	-7.502e-3	3	773.22	1	4782.55	1
511	9	max	.002	1	.07	3	.02	3	1.317e-2	2	NC	5	NC	1
512		min	0	3	-.128	2	-.018	2	-8.181e-3	3	1462.833	1	NC	1
513	10	max	.002	1	.053	3	.018	3	1.428e-2	2	NC	4	NC	1
514		min	0	3	-.087	2	-.024	2	-8.861e-3	3	2464.399	1	8732.938	2
515	11	max	.002	1	.07	3	.017	3	1.317e-2	2	NC	5	NC	1
516		min	0	3	-.128	2	-.018	2	-8.181e-3	3	1462.834	1	NC	1
517	12	max	.002	1	.109	3	.027	1	1.206e-2	2	NC	5	NC	2
518		min	0	3	-.219	1	-.008	10	-7.5e-3	3	773.22	1	4781.584	1
519	13	max	.002	1	.15	3	.059	1	1.094e-2	2	NC	5	NC	3
520		min	0	3	-.318	1	-.004	10	-6.82e-3	3	511.253	1	2360.792	1
521	14	max	.002	1	.18	3	.081	1	9.83e-3	2	NC	5	NC	3
522		min	0	3	-.394	1	-.001	10	-6.14e-3	3	406.368	1	1754.283	1
523	15	max	.002	1	.192	3	.087	1	8.717e-3	2	NC	5	NC	3
524		min	0	3	-.425	1	0	10	-5.459e-3	3	375.232	1	1644.806	1
525	16	max	.002	1	.18	3	.075	1	7.605e-3	2	NC	5	NC	3
526		min	0	3	-.399	1	0	10	-4.779e-3	3	400.998	1	1889.119	1
527	17	max	.002	1	.143	3	.049	1	6.492e-3	2	NC	5	NC	3
528		min	0	3	-.316	1	0	10	-4.099e-3	3	515.373	1	2789.191	1
529	18	max	.002	1	.086	3	.018	1	5.379e-3	2	NC	5	NC	2
530		min	0	3	-.185	1	-.002	10	-3.418e-3	3	938.099	1	6564.817	1
531	19	max	.002	1	.016	3	.006	3	4.267e-3	2	NC	1	NC	1
532		min	0	3	-.026	2	-.007	2	-2.738e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.201e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.109e-5	2	NC	1	NC	1
535	2	max	0	3	-.002	15	.001	1	8.146e-4	3	NC	1	NC	1
536		min	0	2	-.01	4	0	3	-6.393e-4	1	8402.101	4	NC	1
537	3	max	0	3	-.005	15	.004	1	1.309e-3	3	NC	5	NC	1
538		min	0	2	-.019	4	-.003	3	-1.226e-3	1	4275.543	4	NC	1
539	4	max	0	3	-.007	15	.008	1	1.804e-3	3	NC	15	NC	4
540		min	0	2	-.028	4	-.007	3	-1.812e-3	1	2933.272	4	7336.254	1
541	5	max	0	3	-.008	15	.012	1	2.298e-3	3	9737.147	15	NC	4
542		min	0	2	-.036	4	-.011	3	-2.398e-3	1	2288.862	4	4838.526	1
543	6	max	0	3	-.01	15	.017	1	2.793e-3	3	8194.839	15	NC	4
544		min	0	2	-.043	4	-.015	3	-2.984e-3	1	1926.319	4	3534.957	1
545	7	max	0	3	-.011	15	.023	1	3.287e-3	3	7267.343	15	NC	4
546		min	0	2	-.048	4	-.02	3	-3.57e-3	1	1708.298	4	2770.212	1
547	8	max	0	3	-.012	15	.028	1	3.782e-3	3	6710.707	15	NC	4
548		min	0	2	-.052	4	-.025	3	-4.157e-3	1	1577.452	4	2288.389	1
549	9	max	0	3	-.013	15	.032	1	4.276e-3	3	6411.092	15	NC	4
550		min	0	2	-.055	4	-.029	3	-4.743e-3	1	1507.023	4	1972.637	1
551	10	max	0	3	-.013	15	.036	1	4.771e-3	3	6316.308	15	NC	5
552		min	0	2	-.056	4	-.032	3	-5.329e-3	1	1484.743	4	1764.118	1
553	11	max	0	3	-.013	15	.039	1	5.266e-3	3	6411.092	15	NC	5
554		min	0	2	-.055	4	-.035	3	-5.915e-3	1	1507.023	4	1631.91	1
555	12	max	0	3	-.012	15	.04	1	5.76e-3	3	6710.707	15	NC	5
556		min	-.001	2	-.053	4	-.036	3	-6.501e-3	1	1577.452	4	1560.704	1
557	13	max	0	3	-.011	15	.039	1	6.255e-3	3	7267.343	15	NC	5
558		min	-.001	2	-.049	4	-.035	3	-7.088e-3	1	1708.298	4	1546.169	1
559	14	max	0	3	-.01	15	.036	1	6.749e-3	3	8194.839	15	NC	5
560		min	-.001	2	-.043	4	-.032	3	-7.674e-3	1	1926.319	4	1595.204	1
561	15	max	0	3	-.009	15	.031	1	7.244e-3	3	9737.147	15	NC	4
562		min	-.001	2	-.037	4	-.028	3	-8.26e-3	1	2288.862	4	1732.725	1
563	16	max	0	3	-.007	15	.023	1	7.738e-3	3	NC	15	NC	4
564		min	-.001	2	-.029	4	-.02	3	-8.846e-3	1	2933.272	4	2026.219	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	3	-0.005	15	.012	1	8.233e-3	3	NC	5	NC	4
566		min	-0.002	2	-.02	4	-.01	3	-9.432e-3	1	4275.543	4	2687.281	1
567	18	max	0	3	-0.002	12	.004	3	8.727e-3	3	NC	1	NC	4
568		min	-0.002	2	-.011	4	-.008	2	-1.002e-2	1	8402.101	4	4786.146	1
569	19	max	0	3	.004	3	.021	3	9.222e-3	3	NC	1	NC	1
570		min	-0.002	2	-.003	1	-.025	2	-1.06e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.007	3	2.739e-3	3	NC	1	NC	1
572		min	0	3	-.001	1	-.007	2	-2.837e-3	1	NC	1	NC	1
573	2	max	0	10	-0.002	15	.003	1	2.622e-3	3	NC	1	NC	1
574		min	0	3	-.01	4	-.001	10	-2.704e-3	1	8402.101	4	NC	1
575	3	max	0	10	-0.005	15	.009	1	2.505e-3	3	NC	5	NC	4
576		min	0	3	-.02	4	-.004	3	-2.57e-3	1	4275.543	4	6095.921	1
577	4	max	0	10	-0.007	15	.014	1	2.389e-3	3	NC	15	NC	4
578		min	0	3	-.028	4	-.007	3	-2.436e-3	1	2933.272	4	4631.484	1
579	5	max	0	10	-0.008	15	.017	1	2.272e-3	3	9737.147	15	NC	4
580		min	0	3	-.036	4	-.01	3	-2.303e-3	1	2288.862	4	3995.086	1
581	6	max	0	10	-.01	15	.019	1	2.155e-3	3	8194.839	15	NC	4
582		min	0	3	-.043	4	-.011	3	-2.169e-3	1	1926.319	4	3714.767	1
583	7	max	0	10	-0.011	15	.019	1	2.039e-3	3	7267.343	15	NC	4
584		min	0	3	-.048	4	-.012	3	-2.041e-3	2	1708.298	4	3642.343	1
585	8	max	0	10	-0.012	15	.019	1	1.922e-3	3	6710.707	15	NC	4
586		min	0	3	-.052	4	-.012	3	-1.914e-3	2	1577.452	4	3726.682	1
587	9	max	0	10	-0.013	15	.018	1	1.805e-3	3	6411.092	15	NC	4
588		min	0	3	-.055	4	-.011	3	-1.787e-3	2	1507.023	4	3959.992	1
589	10	max	0	10	-0.013	15	.016	1	1.689e-3	3	6316.308	15	NC	4
590		min	0	3	-.056	4	-.01	3	-1.66e-3	2	1484.743	4	4365.098	1
591	11	max	0	10	-0.013	15	.014	1	1.572e-3	3	6411.092	15	NC	4
592		min	0	3	-.055	4	-.009	3	-1.533e-3	2	1507.023	4	5000.061	1
593	12	max	0	10	-0.012	15	.012	1	1.455e-3	3	6710.707	15	NC	4
594		min	0	3	-.052	4	-.007	3	-1.406e-3	2	1577.452	4	5980.2	1
595	13	max	0	10	-0.011	15	.009	1	1.339e-3	3	7267.343	15	NC	2
596		min	0	3	-.048	4	-.006	3	-1.279e-3	2	1708.298	4	7534.52	1
597	14	max	0	10	-.01	15	.007	1	1.222e-3	3	8194.839	15	NC	1
598		min	0	3	-.043	4	-.004	3	-1.152e-3	2	1926.319	4	NC	1
599	15	max	0	10	-0.008	15	.004	1	1.105e-3	3	9737.147	15	NC	1
600		min	0	3	-.036	4	-.002	3	-1.025e-3	2	2288.862	4	NC	1
601	16	max	0	10	-0.007	15	.002	1	9.885e-4	3	NC	15	NC	1
602		min	0	3	-.028	4	-.001	3	-8.978e-4	2	2933.272	4	NC	1
603	17	max	0	10	-0.005	15	0	1	8.718e-4	3	NC	5	NC	1
604		min	0	3	-.019	4	0	10	-7.708e-4	2	4275.543	4	NC	1
605	18	max	0	10	-0.002	15	0	4	7.551e-4	3	NC	1	NC	1
606		min	0	3	-.01	4	0	2	-6.437e-4	2	8402.101	4	NC	1
607	19	max	0	1	0	1	0	1	6.384e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.167e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

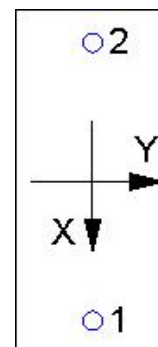
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.