



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

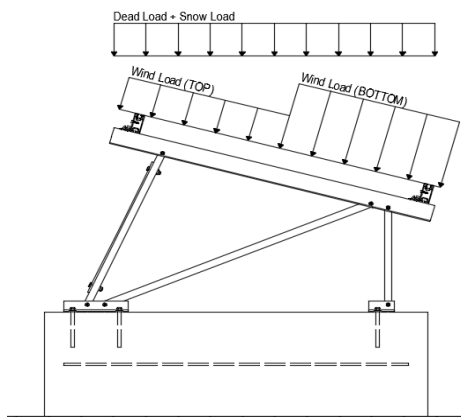
1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	110 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 19.00 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

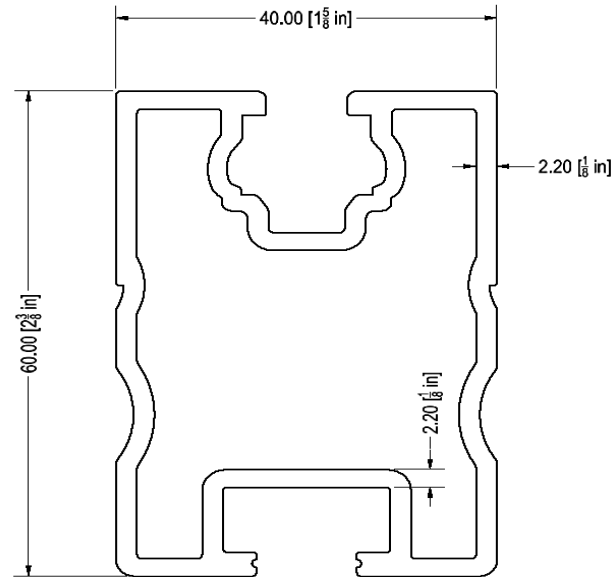
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	54 in
ΦF_{ty} STRONG-AXIS =	29.52 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.458 k-ft
M_z =	0.044 k-ft
$M_{y \text{ allowable}}$ =	1.256 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	42%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.61 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.589 k-ft
M_z =	0.000 k-ft
P_n =	0.272 k
$M_{y \text{ allowable}}$ =	1.453 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	43%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.030 k-ft
P_n =	0.151 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

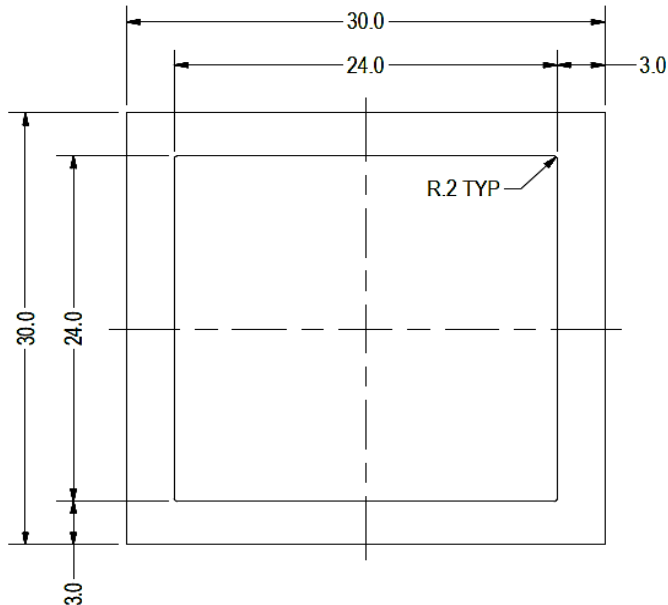
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.757 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

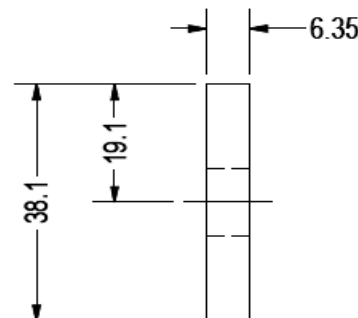
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.634 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	14%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.188 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

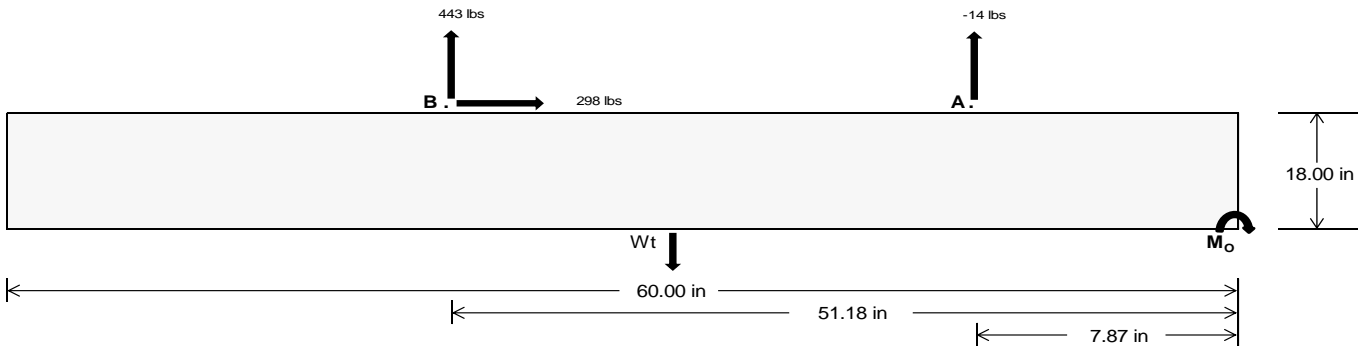
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	5.36	1845.55	k
Compressive Load =	922.68	1233.36	k
Lateral Load =	24.61	1238.88	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27932.1$ in-lbs
Resisting Force Required = 931.07 lbs
S.F. = 1.67
Weight Required = 1551.78 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 297.72 lbs
Friction = 0.4
Weight Required = 744.29 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 297.72 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	326 lbs	326 lbs	326 lbs	326 lbs	335 lbs	335 lbs	335 lbs	335 lbs	463 lbs	463 lbs	463 lbs	463 lbs	28 lbs	28 lbs	28 lbs	28 lbs
F_B	213 lbs	213 lbs	213 lbs	213 lbs	552 lbs	552 lbs	552 lbs	552 lbs	551 lbs	551 lbs	551 lbs	551 lbs	-886 lbs	-886 lbs	-886 lbs	-886 lbs
F_V	35 lbs	35 lbs	35 lbs	35 lbs	538 lbs	538 lbs	538 lbs	538 lbs	426 lbs	426 lbs	426 lbs	426 lbs	-595 lbs	-595 lbs	-595 lbs	-595 lbs
P_{total}	2533 lbs	2623 lbs	2714 lbs	2805 lbs	2880 lbs	2971 lbs	3062 lbs	3152 lbs	3007 lbs	3098 lbs	3188 lbs	3279 lbs	338 lbs	392 lbs	447 lbs	501 lbs
M	279 lbs-ft	279 lbs-ft	279 lbs-ft	279 lbs-ft	450 lbs-ft	450 lbs-ft	450 lbs-ft	450 lbs-ft	520 lbs-ft	520 lbs-ft	520 lbs-ft	520 lbs-ft	724 lbs-ft	724 lbs-ft	724 lbs-ft	724 lbs-ft
e	0.11 ft	0.11 ft	0.10 ft	0.10 ft	0.16 ft	0.15 ft	0.15 ft	0.14 ft	0.17 ft	0.17 ft	0.17 ft	0.16 ft	0.16 ft	2.14 ft	1.84 ft	1.62 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	239.8 psf	238.8 psf	237.9 psf	237.1 psf	255.3 psf	253.6 psf	252.1 psf	250.7 psf	259.9 psf	258.1 psf	256.4 psf	254.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	312.8 psf	308.7 psf	304.9 psf	301.4 psf	373.2 psf	366.4 psf	360.2 psf	354.5 psf	396.2 psf	388.4 psf	381.3 psf	374.7 psf	341.8 psf	208.0 psf	169.1 psf	151.8 psf

Maximum Bearing Pressure = 396 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

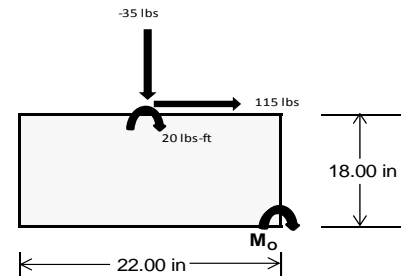
Overturning Check

$M_o = 223.4$ ft-lbs
 Resisting Force Required = 243.73 lbs
 S.F. = 1.67
 Weight Required = 406.22 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	127 lbs	64 lbs	62 lbs	219 lbs	396 lbs	169 lbs	86 lbs	-35 lbs	23 lbs
F_v	14 lbs	115 lbs	14 lbs	10 lbs	86 lbs	11 lbs	14 lbs	115 lbs	14 lbs
P_{total}	2595 lbs	2532 lbs	2530 lbs	2569 lbs	2745 lbs	2519 lbs	807 lbs	687 lbs	744 lbs
M	39 lbs-ft	192 lbs-ft	41 lbs-ft	28 lbs-ft	144 lbs-ft	31 lbs-ft	39 lbs-ft	192 lbs-ft	41 lbs-ft
e	0.02 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.28 ft	0.06 ft
$L/6$	0.31 ft	1.68 ft	1.80 ft	1.81 ft	1.73 ft	1.81 ft	1.74 ft	1.28 ft	1.72 ft
f_{min}	269.2 sqft	207.7 sqft	261.3 sqft	270.3 sqft	247.9 sqft	263.6 sqft	74.0 sqft	6.5 sqft	66.6 sqft
f_{max}	297.1 psf	344.7 psf	290.7 psf	290.1 psf	351.1 psf	285.9 psf	102.1 psf	143.4 psf	95.9 psf



Maximum Bearing Pressure = 351 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.686 k
Allowable Uplift =	1.214 k
Utilization =	<u>56%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.104 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.710 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>

Diagonal Strut

Maximum Axial Load =	0.757 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>13%</u>



Rear Strut

Maximum Axial Load =	1.124 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.188 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.062 in
	<u>0.062 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &22.2924 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.22 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.453 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	201.708	2	-.031	15	.152	1	0	10	0	4	0	15
30			min	-365.626	3	-.128	4	-.572	5	0	4	0	3	0	6
31		16	max	201.843	2	-.044	15	.152	1	0	10	0	4	0	15
32			min	-365.524	3	-.186	4	-.695	5	0	4	0	3	0	6
33		17	max	201.978	2	-.058	15	.152	1	0	10	0	4	0	15
34			min	-365.423	3	-.243	4	-.818	5	0	4	0	3	0	6
35		18	max	202.113	2	-.071	15	.152	1	0	10	0	1	0	15
36			min	-365.322	3	-.301	4	-.941	5	0	4	0	3	0	6
37		19	max	202.248	2	-.085	15	.152	1	0	10	0	1	0	15
38			min	-365.221	3	-.358	4	-1.064	5	0	4	0	3	0	6
39	M3	1	max	226.511	2	1.734	6	-.007	10	0	5	0	1	0	6
40			min	-218.94	3	.407	15	-1.33	4	0	1	0	10	0	15
41		2	max	226.441	2	1.558	6	-.007	10	0	5	0	1	0	2
42			min	-218.993	3	.366	15	-1.197	4	0	1	0	10	0	3
43		3	max	226.371	2	1.381	6	-.007	10	0	5	0	1	0	2
44			min	-219.045	3	.324	15	-1.063	4	0	1	0	5	0	3
45		4	max	226.301	2	1.205	6	-.007	10	0	5	0	1	0	15
46			min	-219.098	3	.283	15	-.929	4	0	1	0	5	0	4
47		5	max	226.231	2	1.029	6	-.007	10	0	5	0	1	0	15
48			min	-219.15	3	.241	15	-.796	4	0	1	0	5	0	4
49		6	max	226.161	2	.852	6	-.007	10	0	5	0	1	0	15
50			min	-219.203	3	.2	15	-.662	4	0	1	0	5	0	4
51		7	max	226.091	2	.676	6	-.007	10	0	5	0	1	0	15
52			min	-219.255	3	.158	15	-.528	4	0	1	0	5	0	4
53		8	max	226.021	2	.499	6	-.007	10	0	5	0	1	0	15
54			min	-219.308	3	.117	15	-.395	4	0	1	0	5	-.001	4
55		9	max	225.951	2	.323	6	-.007	10	0	5	0	1	0	15
56			min	-219.36	3	.075	15	-.261	4	0	1	0	5	-.001	4
57		10	max	225.881	2	.147	6	-.007	10	0	5	0	1	0	15
58			min	-219.413	3	.034	15	-.185	1	0	1	0	5	-.001	4
59		11	max	225.811	2	.005	2	.052	5	0	5	0	1	0	15
60			min	-219.465	3	-.054	3	-.185	1	0	1	0	5	-.001	4
61		12	max	225.741	2	-.049	15	.185	5	0	5	0	1	0	15
62			min	-219.518	3	-.206	4	-.185	1	0	1	0	5	-.001	4
63		13	max	225.671	2	-.091	15	.319	5	0	5	0	1	0	15
64			min	-219.57	3	-.382	4	-.185	1	0	1	0	5	-.001	4
65		14	max	225.601	2	-.132	15	.453	5	0	5	0	1	0	15
66			min	-219.623	3	-.559	4	-.185	1	0	1	0	5	-.001	4
67		15	max	225.531	2	-.173	15	.586	5	0	5	0	1	0	15
68			min	-219.675	3	-.735	4	-.185	1	0	1	0	5	0	4
69		16	max	225.461	2	-.215	15	.72	5	0	5	0	1	0	15
70			min	-219.728	3	-.912	4	-.185	1	0	1	0	5	0	4
71		17	max	225.391	2	-.256	15	.854	5	0	5	0	10	0	15
72			min	-219.78	3	-1.088	4	-.185	1	0	1	0	4	0	4
73		18	max	225.321	2	-.298	15	.987	5	0	5	0	10	0	15
74			min	-219.833	3	-1.264	4	-.185	1	0	1	0	4	0	4
75		19	max	225.251	2	-.339	15	1.121	5	0	5	0	5	0	1
76			min	-219.885	3	-1.441	4	-.185	1	0	1	0	1	0	1
77	M4	1	max	267.764	1	0	1	-.028	10	0	1	0	5	0	1
78			min	21.55	15	0	1	-17.823	4	0	1	0	2	0	1
79		2	max	267.829	1	0	1	-.028	10	0	1	0	10	0	1
80			min	21.569	15	0	1	-17.88	4	0	1	-.002	4	0	1
81		3	max	267.893	1	0	1	-.028	10	0	1	0	10	0	1
82			min	21.589	15	0	1	-17.936	4	0	1	-.003	4	0	1
83		4	max	267.958	1	0	1	-.028	10	0	1	0	10	0	1
84			min	21.608	15	0	1	-17.992	4	0	1	-.005	4	0	1
85		5	max	268.023	1	0	1	-.028	10	0	1	0	10	0	1



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	21.628	15	0	1	-18.048	4	0	1	-.006	4	0	1
87		6	max	268.088	1	0	1	-.028	10	0	1	0	10	0	1
88			min	21.647	15	0	1	-18.104	4	0	1	-.008	4	0	1
89		7	max	268.152	1	0	1	-.028	10	0	1	0	10	0	1
90			min	21.667	15	0	1	-18.16	4	0	1	-.01	4	0	1
91		8	max	268.217	1	0	1	-.028	10	0	1	0	10	0	1
92			min	21.687	15	0	1	-18.216	4	0	1	-.011	4	0	1
93		9	max	268.282	1	0	1	-.028	10	0	1	0	10	0	1
94			min	21.706	15	0	1	-18.272	4	0	1	-.013	4	0	1
95		10	max	268.346	1	0	1	-.028	10	0	1	0	10	0	1
96			min	21.726	15	0	1	-18.328	4	0	1	-.015	4	0	1
97		11	max	268.411	1	0	1	-.028	10	0	1	0	10	0	1
98			min	21.745	15	0	1	-18.384	4	0	1	-.016	4	0	1
99		12	max	268.476	1	0	1	-.028	10	0	1	0	10	0	1
100			min	21.765	15	0	1	-18.44	4	0	1	-.018	4	0	1
101		13	max	268.54	1	0	1	-.028	10	0	1	0	10	0	1
102			min	21.784	15	0	1	-18.496	4	0	1	-.019	4	0	1
103		14	max	268.605	1	0	1	-.028	10	0	1	0	10	0	1
104			min	21.804	15	0	1	-18.552	4	0	1	-.021	4	0	1
105		15	max	268.67	1	0	1	-.028	10	0	1	0	10	0	1
106			min	21.823	15	0	1	-18.609	4	0	1	-.023	4	0	1
107		16	max	268.735	1	0	1	-.028	10	0	1	0	10	0	1
108			min	21.843	15	0	1	-18.665	4	0	1	-.024	4	0	1
109		17	max	268.799	1	0	1	-.028	10	0	1	0	10	0	1
110			min	21.862	15	0	1	-18.721	4	0	1	-.026	4	0	1
111		18	max	268.864	1	0	1	-.028	10	0	1	0	10	0	1
112			min	21.882	15	0	1	-18.777	4	0	1	-.028	4	0	1
113		19	max	268.929	1	0	1	-.028	10	0	1	0	10	0	1
114			min	21.901	15	0	1	-18.833	4	0	1	-.029	4	0	1
115	M6	1	max	631.703	2	.66	6	1.118	4	0	3	0	3	0	1
116			min	-1123.608	3	.146	15	-.247	3	0	5	0	2	0	1
117		2	max	631.838	2	.603	6	.994	4	0	3	0	3	0	15
118			min	-1123.507	3	.132	15	-.247	3	0	5	0	2	0	6
119		3	max	631.973	2	.545	6	.871	4	0	3	0	4	0	15
120			min	-1123.406	3	.119	15	-.247	3	0	5	0	2	0	6
121		4	max	632.108	2	.49	2	.748	4	0	3	0	4	0	15
122			min	-1123.305	3	.105	15	-.247	3	0	5	0	2	0	6
123		5	max	632.243	2	.445	2	.625	4	0	3	0	4	0	15
124			min	-1123.204	3	.092	15	-.247	3	0	5	0	2	0	6
125		6	max	632.377	2	.4	2	.502	4	0	3	0	4	0	15
126			min	-1123.103	3	.075	12	-.247	3	0	5	0	2	0	6
127		7	max	632.512	2	.355	2	.379	4	0	3	0	4	0	15
128			min	-1123.001	3	.052	12	-.247	3	0	5	0	2	0	2
129		8	max	632.647	2	.311	2	.255	4	0	3	.001	4	0	15
130			min	-1122.9	3	.03	12	-.247	3	0	5	0	3	0	2
131		9	max	632.782	2	.266	2	.132	4	0	3	.001	4	0	15
132			min	-1122.799	3	.003	3	-.247	3	0	5	0	3	0	2
133		10	max	632.917	2	.221	2	.03	9	0	3	.001	4	0	15
134			min	-1122.698	3	-.03	3	-.247	3	0	5	0	3	0	2
135		11	max	633.052	2	.176	2	.03	9	0	3	.001	4	0	12
136			min	-1122.597	3	-.064	3	-.247	3	0	5	0	3	0	2
137		12	max	633.187	2	.131	2	.03	9	0	3	.001	4	0	12
138			min	-1122.496	3	-.098	3	-.25	5	0	5	0	3	0	2
139		13	max	633.321	2	.087	2	.03	9	0	3	0	4	0	12
140			min	-1122.395	3	-.131	3	-.373	5	0	5	0	3	0	2
141		14	max	633.456	2	.042	2	.03	9	0	3	0	4	0	12
142			min	-1122.293	3	-.165	3	-.496	5	0	5	0	3	0	2



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	633.591	2	-.003	2	.03	9	0	3	0	4	0	12
144		min	-1122.192	3	-.198	3	-.619	5	0	5	0	3	0	2
145	16	max	633.726	2	-.048	2	.03	9	0	3	0	4	0	12
146		min	-1122.091	3	-.232	3	-.742	5	0	5	0	3	0	2
147	17	max	633.861	2	-.07	15	.03	9	0	3	0	4	0	3
148		min	-1121.99	3	-.265	3	-.866	5	0	5	0	3	0	2
149	18	max	633.996	2	-.084	15	.03	9	0	3	0	4	0	3
150		min	-1121.889	3	-.318	4	-.989	5	0	5	0	3	0	2
151	19	max	634.131	2	-.097	15	.03	9	0	3	0	14	0	3
152		min	-1121.788	3	-.375	4	-1.112	5	0	5	0	3	0	2
153	M7	1	max	756.699	2	1.757	.047	3	0	1	0	4	0	2
154		min	-646.797	3	.421	15	-1.298	4	0	3	0	3	0	3
155	2	max	756.629	2	1.581	4	.047	3	0	1	0	4	0	2
156		min	-646.85	3	.38	15	-1.164	4	0	3	0	3	0	3
157	3	max	756.559	2	1.405	4	.047	3	0	1	0	1	0	2
158		min	-646.902	3	.338	15	-1.03	4	0	3	0	3	0	3
159	4	max	756.489	2	1.228	4	.047	3	0	1	0	1	0	2
160		min	-646.955	3	.297	15	-.897	4	0	3	0	3	0	3
161	5	max	756.419	2	1.052	4	.047	3	0	1	0	1	0	15
162		min	-647.007	3	.255	15	-.763	4	0	3	0	5	0	3
163	6	max	756.349	2	.875	4	.047	3	0	1	0	1	0	15
164		min	-647.06	3	.214	15	-.629	4	0	3	0	5	0	3
165	7	max	756.279	2	.699	4	.047	3	0	1	0	1	0	15
166		min	-647.112	3	.172	15	-.496	4	0	3	0	5	0	6
167	8	max	756.209	2	.523	4	.047	3	0	1	0	1	0	15
168		min	-647.165	3	.126	12	-.362	4	0	3	0	5	-.001	6
169	9	max	756.139	2	.346	4	.047	3	0	1	0	1	0	15
170		min	-647.217	3	.057	12	-.228	4	0	3	0	5	-.001	6
171	10	max	756.069	2	.204	2	.047	3	0	1	0	1	0	15
172		min	-647.27	3	-.024	3	-.095	4	0	3	-.001	5	-.001	6
173	11	max	755.999	2	.066	2	.047	3	0	1	0	1	0	15
174		min	-647.322	3	-.127	3	-.011	1	0	3	-.001	5	-.001	6
175	12	max	755.929	2	-.035	15	.173	5	0	1	0	1	0	15
176		min	-647.375	3	-.23	3	-.011	1	0	3	0	5	-.001	6
177	13	max	755.859	2	-.076	15	.307	5	0	1	0	1	0	15
178		min	-647.427	3	-.36	6	-.011	1	0	3	0	5	-.001	6
179	14	max	755.789	2	-.118	15	.44	5	0	1	0	1	0	15
180		min	-647.48	3	-.536	6	-.011	1	0	3	0	5	-.001	6
181	15	max	755.719	2	-.159	15	.574	5	0	1	0	1	0	15
182		min	-647.532	3	-.713	6	-.011	1	0	3	0	5	0	6
183	16	max	755.649	2	-.201	15	.708	5	0	1	0	1	0	15
184		min	-647.585	3	-.889	6	-.011	1	0	3	0	5	0	6
185	17	max	755.579	2	-.242	15	.841	5	0	1	0	1	0	15
186		min	-647.637	3	-1.065	6	-.011	1	0	3	0	5	0	6
187	18	max	755.509	2	-.284	15	.975	5	0	1	0	1	0	15
188		min	-647.69	3	-1.242	6	-.011	1	0	3	0	3	0	6
189	19	max	755.439	2	-.325	15	1.109	5	0	1	0	1	0	1
190		min	-647.742	3	-1.418	6	-.011	1	0	3	0	3	0	1
191	M8	1	max	708.589	1	0	.244	1	0	1	0	4	0	1
192		min	21.561	15	0	1	-18.053	4	0	1	0	3	0	1
193	2	max	708.654	1	0	1	.244	1	0	1	0	1	0	1
194		min	21.58	15	0	1	-18.109	4	0	1	-.002	4	0	1
195	3	max	708.719	1	0	1	.244	1	0	1	0	1	0	1
196		min	21.6	15	0	1	-18.165	4	0	1	-.003	4	0	1
197	4	max	708.784	1	0	1	.244	1	0	1	0	1	0	1
198		min	21.619	15	0	1	-18.221	4	0	1	-.005	4	0	1
199	5	max	708.848	1	0	1	.244	1	0	1	0	1	0	1





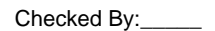
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	203.063	2	-0.008	15	.006	3	0	1	0	5	0	15
258			min	-296.811	3	-.098	3	-.505	4	-.001	5	0	3	0	4
259		16	max	203.198	2	-.021	15	.006	3	0	1	0	5	0	15
260			min	-296.71	3	-.153	6	-.628	4	-.001	5	0	3	0	4
261		17	max	203.333	2	-.035	15	.006	3	0	1	0	5	0	12
262			min	-296.609	3	-.21	6	-.751	4	-.001	5	0	3	0	4
263		18	max	203.468	2	-.049	15	.006	3	0	1	0	5	0	12
264			min	-296.507	3	-.268	6	-.874	4	-.001	5	0	3	0	4
265		19	max	203.602	2	-.062	15	.006	3	0	1	0	5	0	12
266			min	-296.406	3	-.325	6	-.997	4	-.001	5	0	1	0	4
267	M11	1	max	226.032	2	1.72	6	.195	1	0	4	0	5	0	2
268			min	-219.747	3	.397	15	-1.256	5	0	10	0	1	0	15
269		2	max	225.962	2	1.544	6	.195	1	0	4	0	3	0	2
270			min	-219.8	3	.356	15	-1.123	5	0	10	0	1	0	3
271		3	max	225.892	2	1.368	6	.195	1	0	4	0	3	0	2
272			min	-219.852	3	.314	15	-.989	5	0	10	0	1	0	3
273		4	max	225.822	2	1.191	6	.195	1	0	4	0	3	0	15
274			min	-219.905	3	.273	15	-.855	5	0	10	0	1	0	4
275		5	max	225.752	2	1.015	6	.195	1	0	4	0	3	0	15
276			min	-219.957	3	.232	15	-.722	5	0	10	0	1	0	4
277		6	max	225.682	2	.839	6	.195	1	0	4	0	3	0	15
278			min	-220.01	3	.19	15	-.588	5	0	10	0	4	0	4
279		7	max	225.612	2	.662	6	.195	1	0	4	0	3	0	15
280			min	-220.062	3	.149	15	-.454	5	0	10	0	4	-.001	4
281		8	max	225.542	2	.486	6	.195	1	0	4	0	3	0	15
282			min	-220.115	3	.107	15	-.321	5	0	10	0	4	-.001	4
283		9	max	225.472	2	.309	6	.195	1	0	4	0	3	0	15
284			min	-220.167	3	.066	15	-.187	5	0	10	0	4	-.001	4
285		10	max	225.402	2	.143	2	.195	1	0	4	0	3	0	15
286			min	-220.22	3	.024	15	-.061	3	0	10	0	4	-.001	4
287		11	max	225.332	2	.005	2	.195	1	0	4	0	3	0	15
288			min	-220.272	3	-.062	3	-.061	3	0	10	0	4	-.001	4
289		12	max	225.262	2	-.059	15	.262	4	0	4	0	3	0	15
290			min	-220.325	3	-.22	4	-.061	3	0	10	0	4	-.001	4
291		13	max	225.192	2	-.1	15	.396	4	0	4	0	3	0	15
292			min	-220.377	3	-.397	4	-.061	3	0	10	0	4	-.001	4
293		14	max	225.122	2	-.142	15	.529	4	0	4	0	3	0	15
294			min	-220.43	3	-.573	4	-.061	3	0	10	0	4	-.001	4
295		15	max	225.052	2	-.183	15	.663	4	0	4	0	3	0	15
296			min	-220.482	3	-.749	4	-.061	3	0	10	0	4	0	4
297		16	max	224.982	2	-.225	15	.797	4	0	4	0	3	0	15
298			min	-220.535	3	-.926	4	-.061	3	0	10	0	5	0	4
299		17	max	224.912	2	-.266	15	.93	4	0	4	0	3	0	15
300			min	-220.587	3	-1.102	4	-.061	3	0	10	0	10	0	4
301		18	max	224.842	2	-.307	15	1.064	4	0	4	0	3	0	15
302			min	-220.64	3	-1.279	4	-.061	3	0	10	0	10	0	4
303		19	max	224.772	2	-.349	15	1.198	4	0	4	0	4	0	1
304			min	-220.692	3	-1.455	4	-.061	3	0	10	0	10	0	1
305	M12	1	max	267.857	1	0	1	1.194	1	0	1	0	4	0	1
306			min	1.975	15	0	1	-16.577	5	0	1	0	3	0	1
307		2	max	267.922	1	0	1	1.194	1	0	1	0	1	0	1
308			min	1.994	15	0	1	-16.633	5	0	1	-.001	5	0	1
309		3	max	267.986	1	0	1	1.194	1	0	1	0	1	0	1
310			min	2.014	15	0	1	-16.689	5	0	1	-.003	5	0	1
311		4	max	268.051	1	0	1	1.194	1	0	1	0	1	0	1
312			min	2.033	15	0	1	-16.745	5	0	1	-.004	5	0	1
313		5	max	268.116	1	0	1	1.194	1	0	1	0	1	0	1







Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-21.43	2	-30.272	2	-21.578	5	0	5	-.027	1	-.144	3
429	6	max	117.205	3	4.008	9	25.135	1	0	1	.036	5	.115	2
430		min	-21.27	2	-30.501	2	-21.336	5	0	5	-.022	1	-.142	3
431	7	max	117.325	3	3.818	9	25.135	1	0	1	.031	5	.122	2
432		min	-21.11	2	-30.73	2	-21.094	5	0	5	-.016	1	-.14	3
433	8	max	117.445	3	3.627	9	25.135	1	0	1	.027	5	.128	2
434		min	-20.95	2	-30.959	2	-20.852	5	0	5	-.011	1	-.138	3
435	9	max	117.565	3	3.436	9	25.135	1	0	1	.022	5	.135	2
436		min	-20.79	2	-31.187	2	-20.61	5	0	5	-.005	1	-.136	3
437	10	max	117.685	3	3.246	9	25.135	1	0	1	.018	4	.142	2
438		min	-20.629	2	-31.416	2	-20.368	5	0	5	0	1	-.134	3
439	11	max	117.805	3	3.055	9	25.135	1	0	1	.015	4	.149	2
440		min	-20.469	2	-31.645	2	-20.126	5	0	5	0	10	-.132	3
441	12	max	117.926	3	2.865	9	25.135	1	0	1	.012	4	.156	2
442		min	-20.309	2	-31.874	2	-19.884	5	0	5	0	10	-.13	3
443	13	max	118.046	3	2.674	9	25.135	1	0	1	.016	1	.163	2
444		min	-20.149	2	-32.102	2	-19.642	5	0	5	0	10	-.127	3
445	14	max	118.166	3	2.483	9	25.135	1	0	1	.022	1	.17	2
446		min	-19.989	2	-32.331	2	-19.4	5	0	5	0	15	-.125	3
447	15	max	118.286	3	2.293	9	25.135	1	0	1	.027	1	.177	2
448		min	-19.829	2	-32.56	2	-19.158	5	0	5	-.004	5	-.123	3
449	16	max	86.812	2	165.48	2	25.3	1	0	10	.033	1	.182	2
450		min	4.346	15	-205.981	3	-17.766	5	0	4	-.007	5	-.119	3
451	17	max	86.973	2	165.251	2	25.3	1	0	10	.038	1	.146	2
452		min	4.395	15	-206.152	3	-17.524	5	0	4	-.011	5	-.074	3
453	18	max	4.671	5	337.76	2	26.515	1	0	2	.044	1	.074	2
454		min	-89.309	1	-170.349	3	-33.845	5	0	3	-.018	5	-.037	3
455	19	max	4.745	5	337.531	2	26.515	1	0	2	.05	1	0	2
456		min	-89.149	1	-170.52	3	-33.603	5	0	3	-.025	5	0	3
457	M13	1	max	128.983	4	221.011	2	-2.147	15	0	.05	1	0	2
458		min	.979	10	-343.392	3	-89.143	1	0	3	0	15	0	3
459	2	max	124.072	4	157.265	2	-1.167	15	0	2	.015	3	.147	3
460		min	.979	10	-243.708	3	-67.425	1	0	3	-.002	10	-.095	2
461	3	max	119.161	4	93.52	2	-.186	15	0	2	.01	3	.244	3
462		min	.979	10	-144.023	3	-45.706	1	0	3	-.018	1	-.157	2
463	4	max	114.25	4	29.774	2	1.114	5	0	2	.007	3	.291	3
464		min	.979	10	-44.338	3	-23.987	1	0	3	-.035	1	-.188	2
465	5	max	109.34	4	55.347	3	2.631	5	0	2	.004	3	.288	3
466		min	.979	10	-33.971	2	-5.266	3	0	3	-.042	1	-.187	2
467	6	max	104.429	4	155.032	3	19.45	1	0	2	.002	5	.235	3
468		min	.979	10	-97.717	2	-3.839	3	0	3	-.037	1	-.154	2
469	7	max	99.518	4	254.716	3	41.169	1	0	2	.005	5	.133	3
470		min	.979	10	-161.463	2	-2.412	3	0	3	-.022	1	-.089	2
471	8	max	94.607	4	354.401	3	62.888	1	0	2	.008	4	.008	1
472		min	.979	10	-225.208	2	-.986	3	0	3	0	3	-.019	3
473	9	max	89.696	4	454.086	3	84.606	1	0	2	.041	1	.136	2
474		min	.979	10	-288.954	2	.441	3	0	3	-.001	3	-.221	3
475	10	max	84.785	4	553.771	3	106.325	1	0	2	.089	1	.296	2
476		min	.979	10	-352.699	2	1.454	12	0	3	-.011	3	-.473	3
477	11	max	59.53	4	288.954	2	3.221	5	0	3	.04	1	.136	2
478		min	.979	10	-454.086	3	-84.402	1	0	2	-.013	5	-.221	3
479	12	max	54.619	4	225.208	2	4.738	5	0	3	.006	2	.008	1
480		min	.979	10	-354.401	3	-62.683	1	0	2	-.011	5	-.019	3
481	13	max	49.708	4	161.463	2	6.255	5	0	3	0	10	.133	3
482		min	.979	10	-254.716	3	-40.964	1	0	2	-.022	1	-.089	2
483	14	max	44.797	4	97.717	2	7.771	5	0	3	-.003	15	.235	3
484		min	.979	10	-155.032	3	-19.246	1	0	2	-.037	1	-.154	2



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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	39.886	4	33.971	2	10.36	4	0	3	0	15	.288	3
486			min	.979	10	-55.347	3	-1.967	2	0	2	-.041	1	-.187	2
487		16	max	34.976	4	44.338	3	24.192	1	0	3	.005	5	.291	3
488			min	.979	10	-29.774	2	.914	10	0	2	-.035	1	-.188	2
489		17	max	30.065	4	144.023	3	45.91	1	0	3	.01	5	.244	3
490			min	.979	10	-93.52	2	3.622	10	0	2	-.017	1	-.157	2
491		18	max	25.68	1	243.708	3	67.629	1	0	3	.02	4	.147	3
492			min	.979	10	-157.265	2	6.331	10	0	2	-.002	10	-.095	2
493		19	max	25.68	1	343.393	3	89.348	1	0	3	.05	1	0	2
494			min	.979	10	-221.011	2	7.586	12	0	2	.002	10	0	3
495	M16	1	max	33.595	5	337.671	2	4.745	5	0	3	.05	1	0	2
496			min	-26.458	1	-170.553	3	-89.156	1	0	2	-.025	5	0	3
497		2	max	28.685	5	240.018	2	6.262	5	0	3	.011	1	.073	3
498			min	-26.458	1	-121.729	3	-67.437	1	0	2	-.022	5	-.144	2
499		3	max	23.774	5	142.365	2	7.779	5	0	3	0	3	.122	3
500			min	-26.458	1	-72.905	3	-45.719	1	0	2	-.023	4	-.24	2
501		4	max	18.863	5	44.712	2	9.295	5	0	3	-.002	12	.146	3
502			min	-26.458	1	-24.081	3	-.24	1	0	2	-.035	1	-.287	2
503		5	max	13.952	5	24.743	3	10.812	5	0	3	-.003	12	.146	3
504			min	-26.458	1	-52.94	2	-3.527	3	0	2	-.041	1	-.285	2
505		6	max	9.041	5	73.567	3	19.437	1	0	3	-.003	15	.121	3
506			min	-26.458	1	-150.593	2	-2.101	3	0	2	-.037	1	-.234	2
507		7	max	4.13	5	122.391	3	41.156	1	0	3	.003	5	.072	3
508			min	-26.458	1	-248.246	2	-.674	3	0	2	-.022	1	-.134	2
509		8	max	1.975	3	171.215	3	62.875	1	0	3	.01	4	.014	2
510			min	-26.458	1	-345.899	2	.638	12	0	2	-.007	3	-.001	3
511		9	max	1.975	3	220.039	3	84.593	1	0	3	.041	1	.212	2
512			min	-26.458	1	-443.551	2	1.59	12	0	2	-.006	3	-.099	3
513		10	max	19.507	5	-7.752	15	106.312	1	0	14	.089	1	.458	2
514			min	-26.458	1	-541.204	2	-4.813	3	0	2	-.005	3	-.221	3
515		11	max	14.596	5	443.551	2	2.523	5	0	2	.04	1	.212	2
516			min	-26.394	1	-220.039	3	-84.389	1	0	3	-.01	5	-.099	3
517		12	max	9.685	5	345.899	2	4.04	5	0	2	.006	2	.014	2
518			min	-26.394	1	-171.215	3	-62.671	1	0	3	-.008	5	-.001	3
519		13	max	4.774	5	248.246	2	5.556	5	0	2	0	10	.072	3
520			min	-26.394	1	-122.391	3	-40.952	1	0	3	-.022	1	-.134	2
521		14	max	-.044	15	150.593	2	7.073	5	0	2	-.001	12	.121	3
522			min	-26.394	1	-73.567	3	-19.233	1	0	3	-.037	1	-.234	2
523		15	max	-1.007	10	52.94	2	9.64	4	0	2	.001	5	.146	3
524			min	-26.394	1	-24.743	3	-1.929	2	0	3	-.041	1	-.285	2
525		16	max	-1.007	10	24.081	3	24.204	1	0	2	.006	5	.146	3
526			min	-26.394	1	-44.712	2	.935	10	0	3	-.035	1	-.287	2
527		17	max	-1.007	10	72.905	3	45.923	1	0	2	.011	5	.122	3
528			min	-26.394	1	-142.365	2	3.367	12	0	3	-.017	1	-.24	2
529		18	max	-1.007	10	121.729	3	67.642	1	0	2	.021	4	.073	3
530			min	-26.394	1	-240.018	2	4.318	12	0	3	-.002	10	-.144	2
531		19	max	-1.007	10	170.553	3	89.36	1	0	2	.05	1	0	2
532			min	-31.228	4	-337.671	2	5.269	12	0	3	.002	10	0	3
533	M15	1	max	0	1	.939	3	.113	3	0	1	0	1	0	1
534			min	-110.32	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.834	3	.113	3	0	1	0	1	0	1
536			min	-110.395	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.73	3	.113	3	0	1	0	1	0	1
538			min	-110.471	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.626	3	.113	3	0	1	0	1	0	1
540			min	-110.546	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.522	3	.113	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-110.622	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.417	3	.113	3	0	1	0	1	0	1
544		min	-110.697	3	0	1	0	1	0	3	0	3	-.001	3
545	7	max	0	1	.313	3	.113	3	0	1	0	3	0	1
546		min	-110.773	3	0	1	0	1	0	3	0	1	-.001	3
547	8	max	0	1	.209	3	.113	3	0	1	0	3	0	1
548		min	-110.848	3	0	1	0	1	0	3	0	1	-.001	3
549	9	max	0	1	.104	3	.113	3	0	1	0	3	0	1
550		min	-110.924	3	0	1	0	1	0	3	0	1	-.001	3
551	10	max	0	1	0	1	.113	3	0	1	0	3	0	1
552		min	-110.999	3	0	1	0	1	0	3	0	1	-.001	3
553	11	max	0	1	0	1	.113	3	0	1	0	3	0	1
554		min	-111.075	3	-.104	3	0	1	0	3	0	1	-.001	3
555	12	max	0	1	0	1	.113	3	0	1	0	3	0	1
556		min	-111.151	3	-.209	3	0	1	0	3	0	1	-.001	3
557	13	max	0	1	0	1	.113	3	0	1	0	3	0	1
558		min	-111.226	3	-.313	3	0	1	0	3	0	1	-.001	3
559	14	max	0	1	0	1	.113	3	0	1	0	3	0	1
560		min	-111.302	3	-.417	3	0	1	0	3	0	1	-.001	3
561	15	max	0	1	0	1	.113	3	0	1	0	3	0	1
562		min	-111.377	3	-.522	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.113	3	0	1	0	3	0	1
564		min	-111.453	3	-.626	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.113	3	0	1	0	3	0	1
566		min	-111.528	3	-.73	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.113	3	0	1	0	3	0	1
568		min	-111.604	3	-.834	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.113	3	0	1	0	3	0	1
570		min	-111.679	3	-.939	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.369	.35	4	0	3	0	3	0	1
572		min	-188.402	4	0	2	-.047	3	0	1	0	4	0	1
573	2	max	0	2	2.106	4	.315	4	0	3	0	3	0	2
574		min	-188.39	4	0	2	-.047	3	0	1	0	4	0	4
575	3	max	0	2	1.842	4	.279	4	0	3	0	3	0	2
576		min	-188.377	4	0	2	-.047	3	0	1	0	4	-.001	4
577	4	max	0	2	1.579	4	.243	4	0	3	0	3	0	2
578		min	-188.365	4	0	2	-.047	3	0	1	0	1	-.002	4
579	5	max	0	2	1.316	4	.207	4	0	3	0	3	0	2
580		min	-188.353	4	0	2	-.047	3	0	1	0	1	-.002	4
581	6	max	0	2	1.053	4	.171	4	0	3	0	3	0	2
582		min	-188.341	4	0	2	-.047	3	0	1	0	1	-.003	4
583	7	max	0	2	.79	4	.136	4	0	3	0	5	0	2
584		min	-188.328	4	0	2	-.047	3	0	1	0	1	-.003	4
585	8	max	0	2	.526	4	.1	4	0	3	0	5	0	2
586		min	-188.316	4	0	2	-.047	3	0	1	0	1	-.003	4
587	9	max	0	2	.263	4	.064	4	0	3	0	5	0	2
588		min	-188.304	4	0	2	-.047	3	0	1	0	1	-.003	4
589	10	max	0	2	0	1	.032	1	0	3	0	5	0	2
590		min	-188.292	4	0	1	-.047	3	0	1	0	1	-.003	4
591	11	max	0	2	0	2	.032	1	0	3	0	5	0	2
592		min	-188.28	4	-.263	4	-.047	3	0	1	0	1	-.003	4
593	12	max	0	2	0	2	.032	1	0	3	0	5	0	2
594		min	-188.267	4	-.526	4	-.047	3	0	1	0	1	-.003	4
595	13	max	.009	11	0	2	.032	1	0	3	0	5	0	2
596		min	-188.255	4	-.79	4	-.082	5	0	1	0	3	-.003	4
597	14	max	.093	11	0	2	.032	1	0	3	0	5	0	2
598		min	-188.243	4	-1.053	4	-.118	5	0	1	0	3	-.003	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.177	11	0	2	.032	1	0	3	0	5	0	2
600		min	-188.231	4	-1.316	4	-.153	5	0	1	0	3	-.002	4
601	16	max	.261	11	0	2	.032	1	0	3	0	1	0	2
602		min	-188.218	4	-1.579	4	-.189	5	0	1	0	3	-.002	4
603	17	max	.345	11	0	2	.032	1	0	3	0	1	0	2
604		min	-188.206	4	-1.842	4	-.225	5	0	1	0	3	-.001	4
605	18	max	.429	11	0	2	.032	1	0	3	0	1	0	2
606		min	-188.194	4	-2.106	4	-.261	5	0	1	0	5	0	4
607	19	max	.513	11	0	2	.032	1	0	3	0	1	0	1
608		min	-188.201	5	-2.369	4	-.296	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	2	.011	2	.004	1	1.035e-3	5	NC	3	NC	2	
2			min	-.004	3	-.011	3	-.011	5	-4.197e-4	1	3966.91	2	9501.99	1	
3			2	max	.002	2	.01	2	.004	1	1.057e-3	5	NC	3	NC	1
4				min	-.004	3	-.011	3	-.011	5	-4.008e-4	1	4339.717	2	NC	1
5			3	max	.002	2	.009	2	.004	1	1.078e-3	5	NC	3	NC	1
6				min	-.003	3	-.01	3	-.011	5	-3.818e-4	1	4784.995	2	NC	1
7			4	max	.002	2	.008	2	.004	1	1.1e-3	5	NC	1	NC	1
8				min	-.003	3	-.01	3	-.011	5	-3.629e-4	1	5320.51	2	NC	1
9			5	max	.002	2	.007	2	.003	1	1.121e-3	5	NC	1	NC	1
10				min	-.003	3	-.009	3	-.01	5	-3.439e-4	1	5970.015	2	NC	1
11			6	max	.002	2	.006	2	.003	1	1.143e-3	5	NC	1	NC	1
12				min	-.003	3	-.009	3	-.01	5	-3.25e-4	1	6765.79	2	NC	1
13			7	max	.001	2	.005	2	.003	1	1.164e-3	5	NC	1	NC	1
14				min	-.003	3	-.008	3	-.01	5	-3.06e-4	1	7752.526	2	NC	1
15			8	max	.001	2	.005	2	.002	1	1.186e-3	5	NC	1	NC	1
16				min	-.002	3	-.008	3	-.009	5	-2.871e-4	1	8993.48	2	NC	1
17			9	max	.001	2	.004	2	.002	1	1.207e-3	5	NC	1	NC	1
18				min	-.002	3	-.007	3	-.009	5	-2.682e-4	1	NC	1	NC	1
19			10	max	.001	2	.003	2	.002	1	1.228e-3	5	NC	1	NC	1
20				min	-.002	3	-.007	3	-.008	5	-2.492e-4	1	NC	1	NC	1
21		11	max	0	2	.003	2	.001	1	1.25e-3	5	NC	1	NC	1	
22			min	-.002	3	-.006	3	-.008	5	-2.303e-4	1	NC	1	NC	1	
23		12	max	0	2	.002	2	.001	1	1.271e-3	5	NC	1	NC	1	
24			min	-.001	3	-.005	3	-.007	5	-2.113e-4	1	NC	1	NC	1	
25		13	max	0	2	.002	2	0	1	1.293e-3	5	NC	1	NC	1	
26			min	-.001	3	-.005	3	-.006	5	-1.924e-4	1	NC	1	NC	1	
27		14	max	0	2	.001	2	0	1	1.314e-3	5	NC	1	NC	1	
28			min	-.001	3	-.004	3	-.005	5	-1.734e-4	1	NC	1	NC	1	
29		15	max	0	2	0	2	0	1	1.336e-3	5	NC	1	NC	1	
30			min	0	3	-.003	3	-.004	5	-1.545e-4	1	NC	1	NC	1	
31		16	max	0	2	0	2	0	1	1.357e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.003	5	-1.355e-4	1	NC	1	NC	1	
33		17	max	0	2	0	2	0	1	1.379e-3	5	NC	1	NC	1	
34			min	0	3	-.002	3	-.002	5	-1.166e-4	1	NC	1	NC	1	
35		18	max	0	2	0	2	0	1	1.4e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-9.764e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.422e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-7.869e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	3.773e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-6.798e-4	5	NC	1	NC	1	
41		2	max	0	3	0	2	.003	5	4.751e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-6.86e-4	5	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	5.728e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.923e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.01	5	6.705e-5	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	-6.986e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	7.683e-5	1	NC	1	NC	1
48			min	0	2	-.004	3	0	1	-7.049e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.017	4	8.66e-5	1	NC	1	NC	1
50			min	0	2	-.005	3	0	9	-7.111e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.02	4	9.638e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-7.174e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.024	4	1.062e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	9	-7.237e-4	5	NC	1	NC	1
55		9	max	.001	3	.001	2	.027	4	1.159e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	9	-7.299e-4	5	NC	1	NC	1
57		10	max	.001	3	.002	2	.03	4	1.257e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	10	-7.362e-4	5	NC	1	NC	1
59		11	max	.001	3	.002	2	.033	4	1.355e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	10	-7.425e-4	5	NC	1	NC	1
61		12	max	.002	3	.003	2	.036	4	1.452e-4	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	10	-7.487e-4	5	NC	1	NC	1
63		13	max	.002	3	.004	2	.039	4	1.55e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	10	-7.55e-4	5	NC	1	NC	1
65		14	max	.002	3	.004	2	.042	4	1.648e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	10	-7.613e-4	5	NC	1	NC	1
67		15	max	.002	3	.005	2	.044	4	1.746e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	10	-7.675e-4	5	8585.319	2	NC	1
69		16	max	.002	3	.006	2	.047	4	1.843e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	10	-7.738e-4	5	7283.96	2	NC	1
71		17	max	.002	3	.007	2	.049	4	1.941e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	10	-7.801e-4	5	6275.662	2	NC	1
73		18	max	.002	3	.008	2	.052	4	2.039e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	10	-7.863e-4	5	5485.519	2	NC	1
75		19	max	.002	3	.009	2	.054	4	2.137e-4	1	NC	3	NC	1
76			min	-.003	2	-.009	3	0	10	-7.926e-4	5	4860.799	2	NC	1
77	M4	1	max	.001	1	.012	2	0	10	4.34e-3	5	NC	1	NC	2
78			min	0	15	-.011	3	-.057	4	-3.199e-4	1	NC	1	336.573	4
79		2	max	.001	1	.012	2	0	10	4.34e-3	5	NC	1	NC	2
80			min	0	15	-.01	3	-.053	4	-3.199e-4	1	NC	1	366.877	4
81		3	max	.001	1	.011	2	0	10	4.34e-3	5	NC	1	NC	1
82			min	0	15	-.01	3	-.048	4	-3.199e-4	1	NC	1	402.941	4
83		4	max	.001	1	.01	2	0	10	4.34e-3	5	NC	1	NC	1
84			min	0	15	-.009	3	-.043	4	-3.199e-4	1	NC	1	446.283	4
85		5	max	0	1	.01	2	0	10	4.34e-3	5	NC	1	NC	1
86			min	0	15	-.009	3	-.039	4	-3.199e-4	1	NC	1	498.969	4
87		6	max	0	1	.009	2	0	10	4.34e-3	5	NC	1	NC	1
88			min	0	15	-.008	3	-.034	4	-3.199e-4	1	NC	1	563.873	4
89		7	max	0	1	.008	2	0	10	4.34e-3	5	NC	1	NC	1
90			min	0	15	-.007	3	-.03	4	-3.199e-4	1	NC	1	645.089	4
91		8	max	0	1	.008	2	0	10	4.34e-3	5	NC	1	NC	1
92			min	0	15	-.007	3	-.026	4	-3.199e-4	1	NC	1	748.608	4
93		9	max	0	1	.007	2	0	10	4.34e-3	5	NC	1	NC	1
94			min	0	15	-.006	3	-.022	4	-3.199e-4	1	NC	1	883.496	4
95		10	max	0	1	.006	2	0	10	4.34e-3	5	NC	1	NC	1
96			min	0	15	-.006	3	-.018	4	-3.199e-4	1	NC	1	1063.998	4
97		11	max	0	1	.006	2	0	10	4.34e-3	5	NC	1	NC	1
98			min	0	15	-.005	3	-.015	4	-3.199e-4	1	NC	1	1313.579	4
99		12	max	0	1	.005	2	0	10	4.34e-3	5	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	15	-.004	3	-.012	4	-3.199e-4	1	NC	1	1673.165	4
101		max	0	1	.004	2	0	10	4.34e-3	5	NC	1	NC	1
102		min	0	15	-.004	3	-.009	4	-3.199e-4	1	NC	1	2219.522	4
103		max	0	1	.003	2	0	10	4.34e-3	5	NC	1	NC	1
104		min	0	15	-.003	3	-.006	4	-3.199e-4	1	NC	1	3110.984	4
105		max	0	1	.003	2	0	10	4.34e-3	5	NC	1	NC	1
106		min	0	15	-.002	3	-.004	4	-3.199e-4	1	NC	1	4719.516	4
107		max	0	1	.002	2	0	10	4.34e-3	5	NC	1	NC	1
108		min	0	15	-.002	3	-.002	4	-3.199e-4	1	NC	1	8102.488	4
109		max	0	1	.001	2	0	10	4.34e-3	5	NC	1	NC	1
110		min	0	15	-.001	3	-.001	4	-3.199e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	4.34e-3	5	NC	1	NC	1
112		min	0	15	0	3	0	4	-3.199e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	4.34e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-3.199e-4	1	NC	1	NC	1
115	M6	max	.007	2	.035	2	.001	9	1.107e-3	4	NC	3	NC	1
116		min	-.012	3	-.034	3	-.011	5	-2.006e-7	1	1208.196	2	6362.921	3
117		max	.006	2	.033	2	.001	9	1.128e-3	4	NC	3	NC	1
118		min	-.011	3	-.032	3	-.011	5	-2.183e-6	1	1294.286	2	6725.687	3
119		max	.006	2	.03	2	.001	9	1.15e-3	4	NC	3	NC	1
120		min	-.01	3	-.03	3	-.011	5	-4.165e-6	1	1393.123	2	7160.711	3
121		max	.006	2	.028	2	.001	9	1.171e-3	4	NC	3	NC	1
122		min	-.01	3	-.029	3	-.011	5	-6.147e-6	1	1507.253	2	7682.801	3
123		max	.005	2	.026	2	.001	9	1.192e-3	4	NC	3	NC	1
124		min	-.009	3	-.027	3	-.011	5	-8.129e-6	1	1639.96	2	8311.713	3
125		max	.005	2	.024	2	0	9	1.214e-3	4	NC	3	NC	1
126		min	-.008	3	-.025	3	-.01	5	-1.011e-5	1	1795.54	2	9074.133	3
127		max	.004	2	.021	2	0	9	1.235e-3	4	NC	3	NC	1
128		min	-.008	3	-.023	3	-.01	5	-1.209e-5	1	1979.723	2	NC	1
129		max	.004	2	.019	2	0	1	1.257e-3	4	NC	3	NC	1
130		min	-.007	3	-.021	3	-.01	5	-1.407e-5	1	2200.322	2	NC	1
131		max	.004	2	.017	2	0	1	1.278e-3	4	NC	3	NC	1
132		min	-.007	3	-.019	3	-.009	5	-1.606e-5	1	2468.273	2	NC	1
133		max	.003	2	.015	2	0	1	1.3e-3	4	NC	3	NC	1
134		min	-.006	3	-.018	3	-.009	5	-1.804e-5	1	2799.363	2	NC	1
135		max	.003	2	.013	2	0	1	1.321e-3	4	NC	3	NC	1
136		min	-.005	3	-.016	3	-.008	5	-2.002e-5	1	3217.257	2	NC	1
137		max	.003	2	.011	2	0	1	1.343e-3	4	NC	3	NC	1
138		min	-.005	3	-.014	3	-.007	5	-2.2e-5	1	3759.115	2	NC	1
139		max	.002	2	.009	2	0	1	1.364e-3	4	NC	3	NC	1
140		min	-.004	3	-.012	3	-.006	5	-2.399e-5	1	4486.837	2	NC	1
141		max	.002	2	.008	2	0	1	1.386e-3	4	NC	1	NC	1
142		min	-.003	3	-.01	3	-.005	5	-2.597e-5	1	5511.778	2	NC	1
143		max	.001	2	.006	2	0	1	1.407e-3	4	NC	1	NC	1
144		min	-.003	3	-.008	3	-.004	5	-2.795e-5	1	7056.553	2	NC	1
145		max	.001	2	.004	2	0	1	1.429e-3	4	NC	1	NC	1
146		min	-.002	3	-.006	3	-.003	5	-2.993e-5	1	9640.444	2	NC	1
147		max	0	2	.003	2	0	1	1.45e-3	4	NC	1	NC	1
148		min	-.001	3	-.004	3	-.002	5	-3.191e-5	1	NC	1	NC	1
149		max	0	2	.001	2	0	1	1.472e-3	5	NC	1	NC	1
150		min	0	3	-.002	3	-.001	5	-3.39e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.494e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.588e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.709e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-7.141e-4	5	NC	1	NC	1
155		max	0	3	.001	2	.004	5	1.512e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-7.095e-4	4	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157	3	max	0	3	.003	2	.007	5	1.315e-5	1	NC	1	NC	1
158		min	0	2	-.004	3	0	1	-7.052e-4	4	NC	1	NC	1
159	4	max	.001	3	.004	2	.011	5	1.118e-5	1	NC	1	NC	1
160		min	-.001	2	-.006	3	0	1	-7.009e-4	4	NC	1	NC	1
161	5	max	.002	3	.006	2	.014	5	9.211e-6	1	NC	1	NC	1
162		min	-.002	2	-.008	3	0	1	-6.966e-4	4	8256.579	2	NC	1
163	6	max	.002	3	.007	2	.018	5	2.869e-5	3	NC	1	NC	1
164		min	-.002	2	-.01	3	0	1	-6.923e-4	4	6607.426	2	NC	1
165	7	max	.002	3	.008	2	.021	4	5.468e-5	3	NC	1	NC	1
166		min	-.003	2	-.012	3	0	1	-6.881e-4	4	5481.715	2	NC	1
167	8	max	.003	3	.01	2	.025	4	8.066e-5	3	NC	3	NC	1
168		min	-.003	2	-.014	3	0	1	-6.838e-4	4	4658.161	2	NC	1
169	9	max	.003	3	.011	2	.028	4	1.067e-4	3	NC	3	NC	1
170		min	-.004	2	-.016	3	0	1	-6.795e-4	4	4026.425	2	NC	1
171	10	max	.004	3	.013	2	.031	4	1.326e-4	3	NC	3	NC	1
172		min	-.004	2	-.017	3	0	1	-6.752e-4	4	3525.295	2	NC	1
173	11	max	.004	3	.015	2	.034	4	1.586e-4	3	NC	3	NC	1
174		min	-.005	2	-.019	3	0	1	-6.709e-4	4	3117.986	2	NC	1
175	12	max	.004	3	.017	2	.037	4	1.846e-4	3	NC	3	NC	1
176		min	-.005	2	-.02	3	0	1	-6.667e-4	4	2780.947	2	NC	1
177	13	max	.005	3	.018	2	.04	4	2.106e-4	3	NC	3	NC	1
178		min	-.006	2	-.021	3	0	1	-6.624e-4	4	2498.297	2	NC	1
179	14	max	.005	3	.02	2	.043	4	2.366e-4	3	NC	3	NC	1
180		min	-.006	2	-.022	3	0	1	-6.581e-4	4	2258.852	2	NC	1
181	15	max	.006	3	.022	2	.045	4	2.626e-4	3	NC	3	NC	1
182		min	-.007	2	-.024	3	0	1	-6.538e-4	4	2054.445	2	NC	1
183	16	max	.006	3	.025	2	.048	4	2.886e-4	3	NC	3	NC	1
184		min	-.007	2	-.025	3	0	1	-6.495e-4	4	1878.93	2	NC	1
185	17	max	.007	3	.027	2	.051	4	3.145e-4	3	NC	3	NC	1
186		min	-.008	2	-.026	3	0	1	-6.453e-4	4	1727.569	2	NC	1
187	18	max	.007	3	.029	2	.053	4	3.405e-4	3	NC	3	NC	1
188		min	-.008	2	-.027	3	0	1	-6.41e-4	4	1596.64	2	NC	1
189	19	max	.007	3	.031	2	.056	4	3.665e-4	3	NC	3	NC	1
190		min	-.009	2	-.028	3	0	1	-6.367e-4	4	1483.171	2	NC	1
191	M8	1	max	.003	1	.041	2	0	4.18e-3	4	NC	1	NC	1
192		min	0	15	-.034	3	-.058	4	-2.759e-4	3	NC	1	332.399	4
193	2	max	.003	1	.038	2	0	1	4.18e-3	4	NC	1	NC	1
194		min	0	15	-.032	3	-.053	4	-2.759e-4	3	NC	1	362.327	4
195	3	max	.003	1	.036	2	0	1	4.18e-3	4	NC	1	NC	1
196		min	0	15	-.03	3	-.049	4	-2.759e-4	3	NC	1	397.944	4
197	4	max	.003	1	.034	2	0	1	4.18e-3	4	NC	1	NC	1
198		min	0	15	-.028	3	-.044	4	-2.759e-4	3	NC	1	440.749	4
199	5	max	.003	1	.032	2	0	1	4.18e-3	4	NC	1	NC	1
200		min	0	15	-.026	3	-.039	4	-2.759e-4	3	NC	1	492.783	4
201	6	max	.002	1	.029	2	0	1	4.18e-3	4	NC	1	NC	1
202		min	0	15	-.024	3	-.035	4	-2.759e-4	3	NC	1	556.885	4
203	7	max	.002	1	.027	2	0	1	4.18e-3	4	NC	1	NC	1
204		min	0	15	-.023	3	-.03	4	-2.759e-4	3	NC	1	637.095	4
205	8	max	.002	1	.025	2	0	1	4.18e-3	4	NC	1	NC	1
206		min	0	15	-.021	3	-.026	4	-2.759e-4	3	NC	1	739.334	4
207	9	max	.002	1	.023	2	0	1	4.18e-3	4	NC	1	NC	1
208		min	0	15	-.019	3	-.022	4	-2.759e-4	3	NC	1	872.554	4
209	10	max	.002	1	.02	2	0	1	4.18e-3	4	NC	1	NC	1
210		min	0	15	-.017	3	-.018	4	-2.759e-4	3	NC	1	1050.824	4
211	11	max	.002	1	.018	2	0	1	4.18e-3	4	NC	1	NC	1
212		min	0	15	-.015	3	-.015	4	-2.759e-4	3	NC	1	1297.32	4
213	12	max	.001	1	.016	2	0	1	4.18e-3	4	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	4.979e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.803e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.449e-5	3	NC	1	NC	1
274			min	0	2	-.003	3	-.001	3	-7.306e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.012	4	-8.094e-7	3	NC	1	NC	1
276			min	0	2	-.004	3	-.002	3	-7.809e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.015	5	-3.414e-6	10	NC	1	NC	1
278			min	0	2	-.005	3	-.002	3	-8.312e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.018	5	-3.897e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-8.816e-4	4	NC	1	NC	1
281		8	max	0	3	.001	2	.021	5	-4.38e-6	10	NC	1	NC	1
282			min	0	2	-.006	3	-.002	3	-9.319e-4	4	NC	1	NC	1
283		9	max	.001	3	.001	2	.023	5	-4.863e-6	10	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	3	-9.822e-4	4	NC	1	NC	1
285		10	max	.001	3	.002	2	.026	5	-5.346e-6	10	NC	1	NC	1
286			min	-.001	2	-.007	3	-.003	3	-1.032e-3	4	NC	1	NC	1
287		11	max	.001	3	.002	2	.029	5	-5.829e-6	10	NC	1	NC	1
288			min	-.001	2	-.008	3	-.003	3	-1.083e-3	4	NC	1	NC	1
289		12	max	.002	3	.003	2	.031	5	-6.312e-6	10	NC	1	NC	1
290			min	-.002	2	-.008	3	-.003	3	-1.133e-3	4	NC	1	NC	1
291		13	max	.002	3	.004	2	.034	5	-6.795e-6	10	NC	1	NC	1
292			min	-.002	2	-.008	3	-.003	3	-1.183e-3	4	NC	1	NC	1
293		14	max	.002	3	.004	2	.037	5	-7.278e-6	10	NC	1	NC	1
294			min	-.002	2	-.009	3	-.003	3	-1.234e-3	4	NC	1	NC	1
295		15	max	.002	3	.005	2	.039	5	-7.761e-6	10	NC	1	NC	1
296			min	-.002	2	-.009	3	-.003	1	-1.284e-3	4	8598.11	2	NC	1
297		16	max	.002	3	.006	2	.041	5	-8.244e-6	10	NC	1	NC	1
298			min	-.002	2	-.009	3	-.003	1	-1.334e-3	4	7293.815	2	NC	1
299		17	max	.002	3	.007	2	.044	5	-8.727e-6	10	NC	1	NC	1
300			min	-.002	2	-.009	3	-.004	1	-1.385e-3	4	6283.46	2	NC	1
301		18	max	.002	3	.008	2	.046	5	-9.21e-6	10	NC	1	NC	1
302			min	-.002	2	-.009	3	-.004	1	-1.435e-3	4	5491.844	2	NC	1
303		19	max	.003	3	.009	2	.049	5	-9.693e-6	10	NC	3	NC	1
304			min	-.003	2	-.009	3	-.005	1	-1.485e-3	4	4866.054	2	NC	1
305	M12	1	max	.001	1	.012	2	.004	1	4.975e-3	4	NC	1	NC	2
306			min	0	15	-.011	3	-.053	5	1.117e-5	10	NC	1	361.429	5
307		2	max	.001	1	.012	2	.003	1	4.975e-3	4	NC	1	NC	2
308			min	0	15	-.01	3	-.049	5	1.117e-5	10	NC	1	393.961	5
309		3	max	.001	1	.011	2	.003	1	4.975e-3	4	NC	1	NC	2
310			min	0	15	-.01	3	-.045	5	1.117e-5	10	NC	1	432.677	5
311		4	max	.001	1	.01	2	.003	1	4.975e-3	4	NC	1	NC	2
312			min	0	15	-.009	3	-.04	5	1.117e-5	10	NC	1	479.204	5
313		5	max	0	1	.01	2	.003	1	4.975e-3	4	NC	1	NC	2
314			min	0	15	-.009	3	-.036	5	1.117e-5	10	NC	1	535.761	5
315		6	max	0	1	.009	2	.002	1	4.975e-3	4	NC	1	NC	2
316			min	0	15	-.008	3	-.032	5	1.117e-5	10	NC	1	605.434	5
317		7	max	0	1	.008	2	.002	1	4.975e-3	4	NC	1	NC	2
318			min	0	15	-.007	3	-.028	5	1.117e-5	10	NC	1	692.613	5
319		8	max	0	1	.008	2	.002	1	4.975e-3	4	NC	1	NC	1
320			min	0	15	-.007	3	-.024	5	1.117e-5	10	NC	1	803.733	5
321		9	max	0	1	.007	2	.001	1	4.975e-3	4	NC	1	NC	1
322			min	0	15	-.006	3	-.02	5	1.117e-5	10	NC	1	948.521	5
323		10	max	0	1	.006	2	.001	1	4.975e-3	4	NC	1	NC	1
324			min	0	15	-.006	3	-.017	5	1.117e-5	10	NC	1	1142.269	5
325		11	max	0	1	.006	2	0	1	4.975e-3	4	NC	1	NC	1
326			min	0	15	-.005	3	-.014	5	1.117e-5	10	NC	1	1410.16	5
327		12	max	0	1	.005	2	0	1	4.975e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	15	-.004	3	-.011	5	1.117e-5	10	NC	1	1796.12	5
329		max	0	1	.004	2	0	1	4.975e-3	4	NC	1	NC	1
330		min	0	15	-.004	3	-.008	5	1.117e-5	10	NC	1	2382.538	5
331		max	0	1	.003	2	0	1	4.975e-3	4	NC	1	NC	1
332		min	0	15	-.003	3	-.006	5	1.117e-5	10	NC	1	3339.346	5
333		max	0	1	.003	2	0	1	4.975e-3	4	NC	1	NC	1
334		min	0	15	-.002	3	-.004	5	1.117e-5	10	NC	1	5065.756	5
335		max	0	1	.002	2	0	1	4.975e-3	4	NC	1	NC	1
336		min	0	15	-.002	3	-.002	5	1.117e-5	10	NC	1	8696.564	5
337		max	0	1	.001	2	0	1	4.975e-3	4	NC	1	NC	1
338		min	0	15	-.001	3	-.001	5	1.117e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	4.975e-3	4	NC	1	NC	1
340		min	0	15	0	3	0	5	1.117e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	4.975e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.117e-5	10	NC	1	NC	1
343	M1	max	.01	3	.027	3	.007	5	7.115e-3	2	NC	1	NC	1
344		min	-.01	2	-.022	2	-.001	9	-1.059e-2	3	NC	1	NC	1
345		max	.01	3	.016	3	.009	5	3.496e-3	2	NC	4	NC	1
346		min	-.01	2	-.013	2	-.003	1	-5.23e-3	3	5263.761	2	NC	1
347		max	.01	3	.007	3	.011	5	3.643e-4	5	NC	4	NC	1
348		min	-.01	2	-.005	2	-.004	1	-2.395e-4	1	2699.676	2	NC	1
349		max	.01	3	.003	2	.014	5	3.723e-4	5	NC	4	NC	1
350		min	-.01	2	-.002	3	-.005	1	-2.055e-4	1	1885.473	2	6563.959	5
351		max	.01	3	.009	2	.017	5	3.803e-4	5	NC	4	NC	1
352		min	-.01	2	-.009	3	-.005	1	-1.715e-4	1	1476.875	3	4662.777	5
353		max	.009	3	.015	2	.02	5	3.883e-4	5	NC	4	NC	1
354		min	-.01	2	-.014	3	-.005	1	-1.375e-4	1	1254.426	3	3561.443	5
355		max	.009	3	.019	2	.023	5	3.963e-4	5	NC	4	NC	1
356		min	-.01	2	-.018	3	-.004	1	-1.035e-4	1	1124.366	3	2853.277	5
357		max	.009	3	.023	2	.027	5	4.043e-4	5	NC	4	NC	1
358		min	-.01	2	-.022	3	-.003	1	-6.945e-5	1	1044.755	2	2365.627	5
359		max	.009	3	.025	2	.03	5	4.122e-4	5	NC	4	NC	1
360		min	-.01	2	-.023	3	-.002	1	-3.664e-5	9	995.897	2	2013.159	5
361		max	.009	3	.026	2	.034	5	4.202e-4	5	NC	4	NC	1
362		min	-.01	2	-.024	3	-.001	9	-1.2e-5	9	976.036	2	1730.818	4
363		max	.009	3	.025	2	.037	4	4.364e-4	4	NC	4	NC	1
364		min	-.01	2	-.023	3	0	9	3.238e-6	10	983.16	2	1515.689	4
365		max	.009	3	.024	2	.041	4	4.532e-4	4	NC	4	NC	1
366		min	-.01	2	-.021	3	0	10	4.404e-6	10	1019.883	2	1349.879	4
367		max	.009	3	.021	2	.045	4	4.699e-4	4	NC	4	NC	1
368		min	-.01	2	-.018	3	0	10	5.57e-6	10	1094.894	2	1219.952	4
369		max	.009	3	.016	2	.048	4	4.866e-4	4	NC	4	NC	1
370		min	-.01	2	-.014	3	0	10	6.736e-6	10	1228.135	2	1116.949	4
371		max	.009	3	.01	2	.052	4	5.034e-4	4	NC	4	NC	1
372		min	-.01	2	-.008	3	0	10	7.902e-6	10	1466.554	2	1034.73	4
373		max	.009	3	.002	2	.055	4	7.306e-4	4	NC	4	NC	1
374		min	-.01	2	-.002	3	0	10	8.742e-6	10	1879.209	3	968.985	4
375		max	.009	3	.006	3	.057	4	5.968e-3	4	NC	4	NC	1
376		min	-.01	2	-.008	2	0	10	-4.447e-5	9	2727.156	3	916.707	4
377		max	.009	3	.014	3	.06	4	5.296e-3	2	NC	1	NC	1
378		min	-.01	2	-.019	2	0	10	-2.816e-3	3	5348.029	3	875.567	4
379		max	.009	3	.023	3	.062	4	1.068e-2	2	NC	1	NC	1
380		min	-.01	2	-.03	2	0	1	-5.756e-3	3	5729.209	2	844.968	4
381	M5	max	.029	3	.085	3	.007	5	1.713e-5	4	NC	1	NC	1
382		min	-.032	2	-.072	2	-.002	9	3.944e-8	11	3719.883	3	NC	1
383		max	.029	3	.051	3	.009	5	1.813e-4	5	NC	4	NC	1
384		min	-.032	2	-.043	2	-.001	9	-2.336e-5	9	1638.029	2	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.029	3	.02	3	.011	5	3.428e-4	5	NC	5	NC	1
386		min	-.032	2	-.016	2	-.001	9	-4.642e-5	9	839.787	2	NC	1
387	4	max	.029	3	.008	2	.014	5	3.592e-4	5	NC	5	NC	1
388		min	-.032	2	-.006	3	-.001	9	-4.439e-5	9	586.184	2	NC	1
389	5	max	.029	3	.029	2	.017	5	3.756e-4	5	NC	5	NC	1
390		min	-.032	2	-.028	3	-.001	9	-4.237e-5	9	463.552	2	8981.809	3
391	6	max	.029	3	.047	2	.021	5	3.919e-4	5	NC	5	NC	1
392		min	-.032	2	-.045	3	-.001	9	-4.034e-5	9	393.661	2	8100.32	3
393	7	max	.029	3	.062	2	.024	5	4.083e-4	5	NC	5	NC	1
394		min	-.032	2	-.059	3	-.001	9	-3.832e-5	9	350.792	2	7690.745	3
395	8	max	.029	3	.073	2	.028	5	4.246e-4	5	NC	5	NC	1
396		min	-.032	2	-.068	3	-.001	9	-3.629e-5	9	324.234	2	7594.433	3
397	9	max	.029	3	.08	2	.032	5	4.41e-4	5	NC	5	NC	1
398		min	-.032	2	-.074	3	-.001	9	-3.427e-5	9	308.986	2	7751.058	3
399	10	max	.028	3	.083	2	.036	5	4.574e-4	5	NC	5	NC	1
400		min	-.032	2	-.075	3	-.001	9	-3.225e-5	9	302.764	2	8152.626	3
401	11	max	.028	3	.082	2	.039	5	4.737e-4	5	NC	5	NC	1
402		min	-.032	2	-.073	3	-.001	9	-3.022e-5	9	304.939	2	8831.83	3
403	12	max	.028	3	.076	2	.043	5	4.901e-4	5	NC	5	NC	1
404		min	-.032	2	-.066	3	0	9	-2.82e-5	9	316.325	2	9868.622	3
405	13	max	.028	3	.066	2	.046	5	5.064e-4	5	NC	5	NC	1
406		min	-.032	2	-.057	3	0	9	-2.617e-5	9	339.63	2	NC	1
407	14	max	.028	3	.051	2	.05	4	5.228e-4	5	NC	5	NC	1
408		min	-.032	2	-.043	3	0	9	-2.415e-5	9	381.075	2	NC	1
409	15	max	.028	3	.031	2	.053	4	5.392e-4	5	NC	5	NC	1
410		min	-.031	2	-.026	3	0	1	-2.212e-5	9	455.333	2	NC	1
411	16	max	.028	3	.006	2	.056	4	7.635e-4	5	NC	5	NC	1
412		min	-.031	2	-.006	3	0	1	-2.182e-5	9	602.816	3	NC	1
413	17	max	.028	3	.019	3	.058	4	5.953e-3	4	NC	5	NC	1
414		min	-.032	2	-.025	2	0	1	-6.369e-5	1	874.509	3	NC	1
415	18	max	.028	3	.045	3	.06	4	3.055e-3	4	NC	4	NC	1
416		min	-.032	2	-.061	2	0	1	-3.264e-5	1	1714.907	3	NC	1
417	19	max	.028	3	.072	3	.062	4	4.632e-6	5	NC	3	NC	1
418		min	-.031	2	-.098	2	0	1	-1.412e-6	3	1754.24	2	NC	1
419	M9	1	max	.01	.026	3	.006	5	1.06e-2	3	NC	1	NC	1
420		min	-.01	2	-.022	2	-.002	9	-7.115e-3	2	NC	1	NC	1
421	2	max	.01	3	.016	3	.005	5	5.204e-3	3	NC	4	NC	1
422		min	-.01	2	-.013	2	0	9	-3.496e-3	2	5264.307	2	NC	1
423	3	max	.01	3	.006	3	.006	4	1.368e-4	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	12	-9.623e-5	3	2697.852	3	NC	1
425	4	max	.01	3	.003	2	.007	4	1.07e-4	1	NC	4	NC	1
426		min	-.01	2	-.002	3	-.001	3	-9.785e-5	3	1830.198	3	NC	1
427	5	max	.01	3	.009	2	.008	4	7.724e-5	1	NC	4	NC	1
428		min	-.01	2	-.009	3	-.003	3	-9.947e-5	3	1441.91	3	8851.373	3
429	6	max	.01	3	.015	2	.01	4	4.748e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.003	3	-1.011e-4	3	1230.91	3	7683.685	3
431	7	max	.01	3	.019	2	.013	4	2.041e-5	4	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-1.027e-4	3	1106.868	3	6545.38	4
433	8	max	.009	3	.023	2	.015	4	3.656e-5	5	NC	4	NC	1
434		min	-.01	2	-.022	3	-.005	3	-1.043e-4	3	1034.086	3	4653.276	4
435	9	max	.009	3	.025	2	.019	4	5.636e-5	5	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-1.059e-4	3	996.008	2	3508.869	4
437	10	max	.009	3	.026	2	.023	5	7.616e-5	5	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-1.076e-4	3	976.134	2	2762.42	4
439	11	max	.009	3	.025	2	.027	5	9.596e-5	5	NC	4	NC	1
440		min	-.01	2	-.023	3	-.005	3	-1.092e-4	3	983.242	2	2247.696	4
441	12	max	.009	3	.024	2	.031	5	1.158e-4	5	NC	4	NC	1



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Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.006	4	-.016	1	-.042	3	-4.734e-3	2	3925.008	1	1070	3
557	13	max	.001	3	.007	5	.032	4	5.652e-3	3	NC	5	5009.985	9
558		min	-.006	4	-.014	9	-.04	3	-5.159e-3	2	4250.578	1	1060.149	3
559	14	max	.001	3	.007	5	.028	2	6.088e-3	3	NC	5	5192.118	9
560		min	-.007	4	-.013	9	-.036	3	-5.585e-3	2	4793.058	1	1093.872	3
561	15	max	.002	3	.007	5	.022	1	6.523e-3	3	NC	5	7668.561	15
562		min	-.007	4	-.011	9	-.029	3	-6.011e-3	2	5695.135	1	1188.27	3
563	16	max	.002	3	.007	5	.015	1	6.959e-3	3	NC	4	NC	13
564		min	-.008	4	-.009	9	-.019	3	-6.436e-3	2	7298.553	1	1389.641	3
565	17	max	.002	3	.007	5	.006	4	7.395e-3	3	NC	1	NC	4
566		min	-.008	4	-.007	9	-.005	3	-6.862e-3	2	NC	1	1843.133	3
567	18	max	.002	3	.006	5	.014	3	7.831e-3	3	NC	1	NC	4
568		min	-.009	4	-.004	9	-.015	2	-7.287e-3	2	NC	1	3282.873	3
569	19	max	.002	3	.009	2	.036	3	8.267e-3	3	NC	1	NC	1
570		min	-.009	4	-.001	9	-.034	2	-7.713e-3	2	NC	1	NC	1
571	M16A	1	max	0	.002	2	.011	3	2.336e-3	3	NC	1	NC	1
572		min	-.003	4	-.004	4	-.011	2	-2.321e-3	2	NC	1	NC	1
573	2	max	0	10	-.001	10	.003	3	2.25e-3	3	NC	1	NC	1
574		min	-.003	4	-.01	4	-.004	2	-2.217e-3	2	NC	1	9240.127	3
575	3	max	0	10	-.004	10	.004	1	2.164e-3	3	NC	1	NC	4
576		min	-.003	4	-.016	4	-.008	5	-2.114e-3	2	5621.47	4	5236.71	3
577	4	max	0	10	-.006	12	.007	1	2.078e-3	3	NC	12	NC	9
578		min	-.003	4	-.021	4	-.014	5	-2.01e-3	2	3856.657	4	3990.431	3
579	5	max	0	10	-.007	12	.009	1	1.992e-3	3	NC	12	NC	9
580		min	-.003	4	-.026	4	-.021	5	-1.906e-3	2	3009.389	4	3453.813	3
581	6	max	0	10	-.008	12	.011	1	1.906e-3	3	9586.116	12	NC	9
582		min	-.002	4	-.03	4	-.028	5	-1.802e-3	2	2532.719	4	2625.833	5
583	7	max	0	10	-.009	12	.011	1	1.82e-3	3	8501.155	12	NC	9
584		min	-.002	4	-.033	4	-.034	5	-1.699e-3	2	2246.064	4	2100.263	5
585	8	max	0	10	-.01	12	.011	1	1.734e-3	3	7850.016	12	NC	9
586		min	-.002	4	-.036	4	-.04	5	-1.595e-3	2	2074.029	4	1800.245	5
587	9	max	0	10	-.01	12	.011	1	1.648e-3	3	7499.534	12	NC	9
588		min	-.002	4	-.037	4	-.043	5	-1.491e-3	2	1981.429	4	1631.378	5
589	10	max	0	10	-.01	12	.01	1	1.562e-3	3	7388.658	12	NC	9
590		min	-.002	4	-.037	4	-.045	5	-1.388e-3	2	1952.135	4	1551.44	5
591	11	max	0	10	-.01	12	.008	1	1.476e-3	3	7499.534	12	NC	9
592		min	-.002	4	-.037	4	-.045	5	-1.284e-3	2	1981.429	4	1543.21	5
593	12	max	0	10	-.009	12	.007	1	1.39e-3	3	7850.016	12	NC	9
594		min	-.001	4	-.035	4	-.044	5	-1.18e-3	2	2074.029	4	1605.406	5
595	13	max	0	10	-.009	12	.005	1	1.304e-3	3	8501.155	12	NC	2
596		min	-.001	4	-.032	4	-.04	5	-1.076e-3	2	2246.064	4	1752.272	5
597	14	max	0	10	-.008	12	.004	1	1.218e-3	3	9586.116	12	NC	1
598		min	0	4	-.028	4	-.035	5	-9.727e-4	2	2532.719	4	2021.632	5
599	15	max	0	10	-.006	12	.002	1	1.132e-3	3	NC	12	NC	1
600		min	0	4	-.024	4	-.028	5	-8.689e-4	2	3009.389	4	2500.687	5
601	16	max	0	10	-.005	12	.001	9	1.046e-3	3	NC	12	NC	1
602		min	0	4	-.019	4	-.021	5	-7.652e-4	2	3856.657	4	3408.89	5
603	17	max	0	10	-.003	12	0	9	9.603e-4	3	NC	1	NC	1
604		min	0	4	-.013	4	-.013	5	-6.615e-4	2	5621.47	4	5440.434	5
605	18	max	0	10	-.002	12	0	3	9.953e-4	4	NC	1	NC	1
606		min	0	4	-.006	4	-.006	5	-5.578e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	1.062e-3	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.541e-4	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

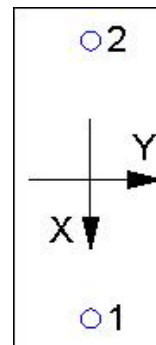
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$ (Eq. D-7)

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ (Sec. D.4.1 & Eq. D-5)

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$ (Eq. D-16f)

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$ (Sec. D.4.1 & Eq. D-16b)

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
 Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.