

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	25° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	150 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 35.33 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.1	(Pressure)
$C_{f+ BOTTOM}$ =	1.7	
$C_{f- TOP}$ =	-2.2	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.028 k-ft
P_n =	0.155 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	8%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.468 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	12%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36.18 in
$\Phi F_{ty \text{ AXIAL}}$ =	11.59 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.23 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.712 k
$M_{y \text{ allowable}}$ =	0.410 k-ft
$M_{z \text{ allowable}}$ =	0.410 k-ft
$P_{n \text{ allowable}}$ =	5.820 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.173 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

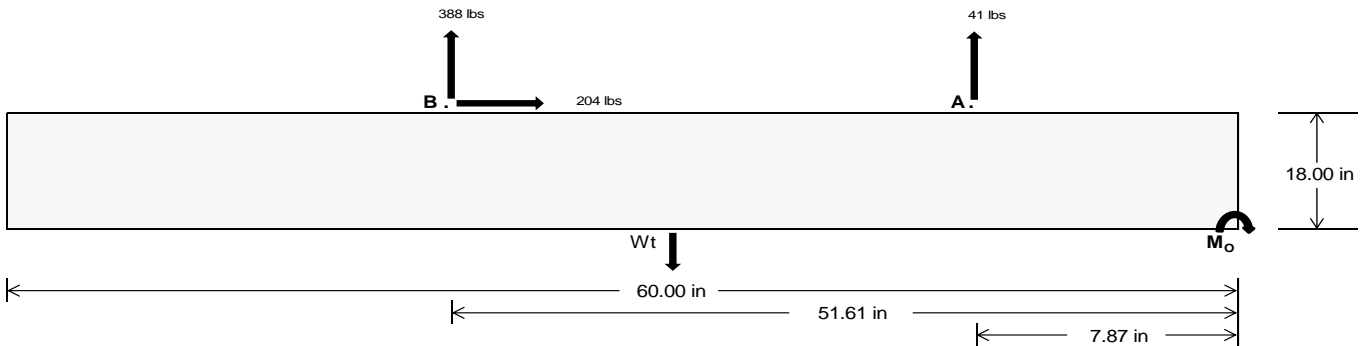
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	183.62	1686.23	k
Compressive Load =	1144.30	1124.58	k
Lateral Load =	22.97	885.47	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 24044.7$ in-lbs
Resisting Force Required = 801.49 lbs
S.F. = 1.67
Weight Required = 1335.81 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 204.28 lbs
Friction = 0.4
Weight Required = 510.69 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 204.28 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	375 lbs	375 lbs	375 lbs	375 lbs	429 lbs	429 lbs	429 lbs	429 lbs	572 lbs	572 lbs	572 lbs	572 lbs	-82 lbs	-82 lbs	-82 lbs	-82 lbs
F_B	263 lbs	263 lbs	263 lbs	263 lbs	466 lbs	466 lbs	466 lbs	466 lbs	524 lbs	524 lbs	524 lbs	524 lbs	-777 lbs	-777 lbs	-777 lbs	-777 lbs
F_V	31 lbs	31 lbs	31 lbs	31 lbs	364 lbs	364 lbs	364 lbs	364 lbs	294 lbs	294 lbs	294 lbs	294 lbs	-409 lbs	-409 lbs	-409 lbs	-409 lbs
P_{total}	2541 lbs	2631 lbs	2722 lbs	2813 lbs	2799 lbs	2890 lbs	2980 lbs	3071 lbs	2999 lbs	3090 lbs	3181 lbs	3271 lbs	283 lbs	337 lbs	392 lbs	446 lbs
M	265 lbs-ft	265 lbs-ft	265 lbs-ft	265 lbs-ft	497 lbs-ft	497 lbs-ft	497 lbs-ft	497 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	551 lbs-ft	634 lbs-ft	634 lbs-ft	634 lbs-ft	634 lbs-ft
e	0.10 ft	0.10 ft	0.10 ft	0.09 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.24 ft	1.88 ft	1.62 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	254.0 psf	252.4 psf	250.9 psf	249.5 psf	251.7 psf	250.2 psf	248.7 psf	247.4 psf	267.2 psf	264.9 psf	262.9 psf	261.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	326.7 psf	321.7 psf	317.2 psf	313.0 psf	388.0 psf	380.3 psf	373.2 psf	366.7 psf	418.4 psf	409.2 psf	400.9 psf	393.3 psf	418.4 psf	198.1 psf	154.8 psf	138.0 psf

Maximum Bearing Pressure = 418 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

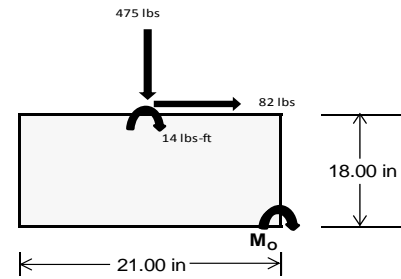
Overturning Check

$M_o = 279.0$ ft-lbs
 Resisting Force Required = 318.88 lbs
 S.F. = 1.67
 Weight Required = 531.47 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	114 lbs	69 lbs	56 lbs	228 lbs	475 lbs	184 lbs	76 lbs	-26 lbs	20 lbs
F_v	13 lbs	109 lbs	13 lbs	9 lbs	82 lbs	10 lbs	13 lbs	109 lbs	13 lbs
P_{total}	2470 lbs	2425 lbs	2412 lbs	2471 lbs	2718 lbs	2427 lbs	765 lbs	663 lbs	709 lbs
M	37 lbs-ft	182 lbs-ft	38 lbs-ft	26 lbs-ft	137 lbs-ft	29 lbs-ft	37 lbs-ft	182 lbs-ft	38 lbs-ft
e	0.01 ft	0.08 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.28 ft	0.05 ft
$L/6$	0.29 ft	1.60 ft	1.72 ft	1.73 ft	1.65 ft	1.73 ft	1.65 ft	1.20 ft	1.64 ft
f_{min}	268.0 sqft	205.6 sqft	260.9 sqft	272.2 sqft	257.0 sqft	265.9 sqft	73.0 sqft	4.3 sqft	66.3 sqft
f_{max}	296.7 psf	348.6 psf	290.5 psf	292.6 psf	364.3 psf	288.7 psf	101.9 psf	147.2 psf	95.8 psf



Maximum Bearing Pressure = 364 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

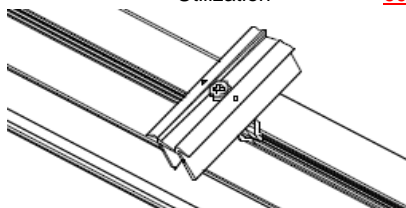
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

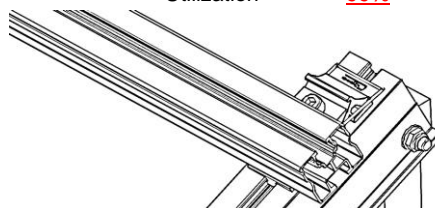
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.732 k
Allowable Uplift =	1.214 k
Utilization =	<u>60%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.109 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

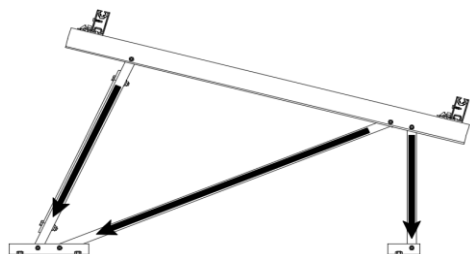
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.880 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>15%</u>

Diagonal Strut

Maximum Axial Load =	0.468 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>8%</u>



Rear Strut

Maximum Axial Load =	1.135 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.173 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

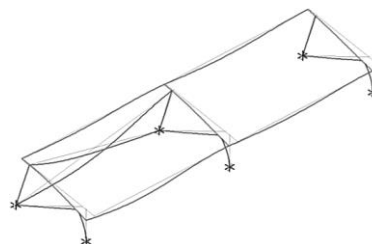
7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	30.83 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.617 in
Max Drift, Δ_{MAX} =	0.058 in
	<u>0.058 ≤ 0.617. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$132.801$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 51.00 \text{ in}$$

$$J = 0.255$$

$$137.906$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.261 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi_c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.33 \\
 &21.3453 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.33 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.461 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.18 \text{ in}$$

$$J = 0.16$$

$$94.9139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.410 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.5514$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7972$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 11.5927 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 11.59 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

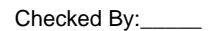
$$0.50 \text{ in}^2$$

$$P_{\max} = 5.82 \text{ kips}$$

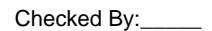
APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 25° 150mph 30psf 4.25ft 7-10Pa Page 20





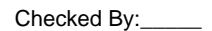
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	226.767	1	.034	2	.153	1	0	10	0	4	0	15
30			min	-365.155	3	-.03	3	-.495	5	0	4	0	3	0	6
31		16	max	226.883	1	-.002	2	.153	1	0	10	0	4	0	15
32			min	-365.068	3	-.057	3	-.601	5	0	4	0	3	0	6
33		17	max	227	1	-.022	15	.153	1	0	10	0	4	0	15
34			min	-364.981	3	-.09	4	-.706	5	0	4	0	3	0	6
35		18	max	227.116	1	-.033	15	.153	1	0	10	0	1	0	15
36			min	-364.893	3	-.135	4	-.812	5	0	4	0	3	0	6
37		19	max	227.233	1	-.044	15	.153	1	0	10	0	1	0	15
38			min	-364.806	3	-.181	4	-.917	5	0	4	0	3	0	6
39	M3	1	max	139.194	2	1.776	6	-.002	10	0	5	0	4	0	6
40			min	-131.821	3	.417	15	-1.343	4	0	1	0	10	0	15
41		2	max	139.126	2	1.599	6	-.002	10	0	5	0	1	0	2
42			min	-131.872	3	.375	15	-1.209	4	0	1	0	10	0	15
43		3	max	139.057	2	1.422	6	-.002	10	0	5	0	1	0	2
44			min	-131.924	3	.333	15	-1.076	4	0	1	0	5	0	3
45		4	max	138.989	2	1.245	6	-.002	10	0	5	0	1	0	15
46			min	-131.975	3	.292	15	-.942	4	0	1	0	5	0	4
47		5	max	138.92	2	1.068	6	-.002	10	0	5	0	1	0	15
48			min	-132.026	3	.25	15	-.809	4	0	1	0	5	0	4
49		6	max	138.851	2	.89	6	-.002	10	0	5	0	1	0	15
50			min	-132.078	3	.208	15	-.675	4	0	1	0	5	0	4
51		7	max	138.783	2	.713	6	-.002	10	0	5	0	1	0	15
52			min	-132.129	3	.167	15	-.541	4	0	1	0	5	0	4
53		8	max	138.714	2	.536	6	-.002	10	0	5	0	1	0	15
54			min	-132.181	3	.125	15	-.408	4	0	1	0	5	-.001	4
55		9	max	138.646	2	.359	6	-.002	10	0	5	0	1	0	15
56			min	-132.232	3	.083	15	-.274	4	0	1	0	5	-.001	4
57		10	max	138.577	2	.182	6	-.002	10	0	5	0	1	0	15
58			min	-132.284	3	.042	15	-.175	1	0	1	0	5	-.001	4
59		11	max	138.508	2	.029	2	.032	5	0	5	0	1	0	15
60			min	-132.335	3	-.022	3	-.175	1	0	1	0	5	-.001	4
61		12	max	138.44	2	-.042	15	.166	5	0	5	0	1	0	15
62			min	-132.387	3	-.173	4	-.175	1	0	1	0	5	-.001	4
63		13	max	138.371	2	-.083	15	.3	5	0	5	0	1	0	15
64			min	-132.438	3	-.35	4	-.175	1	0	1	0	5	-.001	4
65		14	max	138.303	2	-.125	15	.433	5	0	5	0	1	0	15
66			min	-132.49	3	-.527	4	-.175	1	0	1	0	5	-.001	4
67		15	max	138.234	2	-.167	15	.567	5	0	5	0	9	0	15
68			min	-132.541	3	-.705	4	-.175	1	0	1	0	5	0	4
69		16	max	138.165	2	-.208	15	.701	5	0	5	0	10	0	15
70			min	-132.592	3	-.882	4	-.175	1	0	1	0	4	0	4
71		17	max	138.097	2	-.25	15	.834	5	0	5	0	10	0	15
72			min	-132.644	3	-1.059	4	-.175	1	0	1	0	4	0	4
73		18	max	138.028	2	-.292	15	.968	5	0	5	0	10	0	15
74			min	-132.695	3	-1.236	4	-.175	1	0	1	0	4	0	4
75		19	max	137.96	2	-.333	15	1.101	5	0	5	0	5	0	1
76			min	-132.747	3	-1.413	4	-.175	1	0	1	0	1	0	1
77	M4	1	max	308.614	1	0	1	0	10	0	1	0	5	0	1
78			min	-33.296	3	0	1	-16.505	4	0	1	0	2	0	1
79		2	max	308.678	1	0	1	0	10	0	1	0	10	0	1
80			min	-33.247	3	0	1	-16.561	4	0	1	-.001	4	0	1
81		3	max	308.743	1	0	1	0	10	0	1	0	10	0	1
82			min	-33.199	3	0	1	-16.617	4	0	1	-.003	4	0	1
83		4	max	308.808	1	0	1	0	10	0	1	0	10	0	1
84			min	-33.15	3	0	1	-16.673	4	0	1	-.004	4	0	1
85		5	max	308.873	1	0	1	0	10	0	1	0	10	0	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	711.376	1	.097	2	.036	9	0	3	0	4	0	15
144		min	-1133.799	3	-.096	3	-.511	5	0	5	0	3	0	2
145	16	max	711.492	1	.062	2	.036	9	0	3	0	4	0	12
146		min	-1133.712	3	-.122	3	-.616	5	0	5	0	3	0	2
147	17	max	711.609	1	.026	2	.036	9	0	3	0	4	0	12
148		min	-1133.624	3	-.149	3	-.721	5	0	5	0	3	0	2
149	18	max	711.725	1	-.01	2	.036	9	0	3	0	4	0	12
150		min	-1133.537	3	-.176	3	-.827	5	0	5	0	3	0	2
151	19	max	711.841	1	-.045	2	.036	9	0	3	0	14	0	12
152		min	-1133.45	3	-.202	3	-.932	5	0	5	0	3	0	2
153	M7	1	max	467.999	2	1.791	.021	3	0	9	0	4	0	2
154		min	-371.993	3	.426	15	-1.355	4	0	3	0	3	0	12
155	2	max	467.93	2	1.613	4	.021	3	0	9	0	4	0	2
156		min	-372.044	3	.384	15	-1.222	4	0	3	0	3	0	3
157	3	max	467.862	2	1.436	4	.021	3	0	9	0	1	0	2
158		min	-372.095	3	.342	15	-1.088	4	0	3	0	3	0	3
159	4	max	467.793	2	1.259	4	.021	3	0	9	0	1	0	2
160		min	-372.147	3	.301	15	-.954	4	0	3	0	3	0	3
161	5	max	467.724	2	1.082	4	.021	3	0	9	0	1	0	15
162		min	-372.198	3	.259	15	-.821	4	0	3	0	5	0	3
163	6	max	467.656	2	.905	4	.021	3	0	9	0	1	0	15
164		min	-372.25	3	.218	15	-.687	4	0	3	0	5	0	6
165	7	max	467.587	2	.727	4	.021	3	0	9	0	1	0	15
166		min	-372.301	3	.176	15	-.553	4	0	3	0	5	0	6
167	8	max	467.519	2	.55	4	.021	3	0	9	0	1	0	15
168		min	-372.353	3	.134	15	-.42	4	0	3	0	5	0	6
169	9	max	467.45	2	.373	4	.021	3	0	9	0	1	0	15
170		min	-372.404	3	.093	15	-.286	4	0	3	0	5	-.001	6
171	10	max	467.381	2	.217	2	.021	3	0	9	0	1	0	15
172		min	-372.456	3	.025	12	-.153	4	0	3	0	5	-.001	6
173	11	max	467.313	2	.079	2	.021	3	0	9	0	1	0	15
174		min	-372.507	3	-.073	3	-.019	4	0	3	0	5	-.001	6
175	12	max	467.244	2	-.032	15	.116	5	0	9	0	1	0	15
176		min	-372.559	3	-.177	3	-.013	1	0	3	0	5	-.001	6
177	13	max	467.176	2	-.074	15	.249	5	0	9	0	1	0	15
178		min	-372.61	3	-.336	6	-.013	1	0	3	0	5	-.001	6
179	14	max	467.107	2	-.116	15	.383	5	0	9	0	1	0	15
180		min	-372.661	3	-.513	6	-.013	1	0	3	0	5	-.001	6
181	15	max	467.038	2	-.157	15	.517	5	0	9	0	1	0	15
182		min	-372.713	3	-.691	6	-.013	1	0	3	0	5	0	6
183	16	max	466.97	2	-.199	15	.65	5	0	9	0	1	0	15
184		min	-372.764	3	-.868	6	-.013	1	0	3	0	5	0	6
185	17	max	466.901	2	-.241	15	.784	5	0	9	0	1	0	15
186		min	-372.816	3	-1.045	6	-.013	1	0	3	0	5	0	6
187	18	max	466.832	2	-.282	15	.918	5	0	9	0	1	0	15
188		min	-372.867	3	-1.222	6	-.013	1	0	3	0	3	0	6
189	19	max	466.764	2	-.324	15	1.051	5	0	9	0	9	0	1
190		min	-372.919	3	-1.399	6	-.013	1	0	3	0	3	0	1
191	M8	1	max	879.063	1	0	.178	9	0	1	0	4	0	1
192		min	-142.117	3	0	1	-16.799	4	0	1	0	3	0	1
193	2	max	879.128	1	0	1	.178	9	0	1	0	9	0	1
194		min	-142.068	3	0	1	-16.855	4	0	1	-.001	4	0	1
195	3	max	879.192	1	0	1	.178	9	0	1	0	9	0	1
196		min	-142.02	3	0	1	-16.911	4	0	1	-.003	4	0	1
197	4	max	879.257	1	0	1	.178	9	0	1	0	9	0	1
198		min	-141.971	3	0	1	-16.967	4	0	1	-.005	4	0	1
199	5	max	879.322	1	0	1	.178	9	0	1	0	9	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200		min	-141.923	3	0	1	-17.023	4	0	1	-.006	4	0	1
201	6	max	879.386	1	0	1	.178	9	0	1	0	9	0	1
202		min	-141.874	3	0	1	-17.079	4	0	1	-.008	4	0	1
203	7	max	879.451	1	0	1	.178	9	0	1	0	9	0	1
204		min	-141.826	3	0	1	-17.136	4	0	1	-.009	4	0	1
205	8	max	879.516	1	0	1	.178	9	0	1	0	9	0	1
206		min	-141.777	3	0	1	-17.192	4	0	1	-.011	4	0	1
207	9	max	879.58	1	0	1	.178	9	0	1	0	9	0	1
208		min	-141.729	3	0	1	-17.248	4	0	1	-.012	4	0	1
209	10	max	879.645	1	0	1	.178	9	0	1	0	9	0	1
210		min	-141.68	3	0	1	-17.304	4	0	1	-.014	4	0	1
211	11	max	879.71	1	0	1	.178	9	0	1	0	9	0	1
212		min	-141.632	3	0	1	-17.36	4	0	1	-.015	4	0	1
213	12	max	879.775	1	0	1	.178	9	0	1	0	9	0	1
214		min	-141.583	3	0	1	-17.416	4	0	1	-.017	4	0	1
215	13	max	879.839	1	0	1	.178	9	0	1	0	9	0	1
216		min	-141.534	3	0	1	-17.472	4	0	1	-.018	4	0	1
217	14	max	879.904	1	0	1	.178	9	0	1	0	9	0	1
218		min	-141.486	3	0	1	-17.528	4	0	1	-.02	4	0	1
219	15	max	879.969	1	0	1	.178	9	0	1	0	9	0	1
220		min	-141.437	3	0	1	-17.584	4	0	1	-.022	4	0	1
221	16	max	880.033	1	0	1	.178	9	0	1	0	9	0	1
222		min	-141.389	3	0	1	-17.64	4	0	1	-.023	4	0	1
223	17	max	880.098	1	0	1	.178	9	0	1	0	9	0	1
224		min	-141.34	3	0	1	-17.696	4	0	1	-.025	4	0	1
225	18	max	880.163	1	0	1	.178	9	0	1	0	9	0	1
226		min	-141.292	3	0	1	-17.752	4	0	1	-.026	4	0	1
227	19	max	880.228	1	0	1	.178	9	0	1	0	9	0	1
228		min	-141.243	3	0	1	-17.808	4	0	1	-.028	4	0	1
229	M10	1	max	226.777	1	.671	4	1.1	5	0	1	0	1	0
230		min	-312.981	3	.17	15	-.109	1	-.001	5	0	3	0	1
231	2	max	226.893	1	.626	4	.994	5	0	1	0	4	0	15
232		min	-312.894	3	.159	15	-.109	1	-.001	5	0	3	0	4
233	3	max	227.01	1	.58	4	.889	5	0	1	0	4	0	15
234		min	-312.807	3	.148	15	-.109	1	-.001	5	0	3	0	4
235	4	max	227.126	1	.534	4	.783	5	0	1	0	4	0	15
236		min	-312.719	3	.138	15	-.109	1	-.001	5	0	3	0	4
237	5	max	227.243	1	.489	4	.678	5	0	1	0	4	0	15
238		min	-312.632	3	.127	15	-.109	1	-.001	5	0	3	0	4
239	6	max	227.359	1	.443	4	.572	5	0	1	0	4	0	15
240		min	-312.545	3	.116	15	-.109	1	-.001	5	0	3	0	4
241	7	max	227.475	1	.397	4	.467	5	0	1	0	4	0	15
242		min	-312.457	3	.105	15	-.109	1	-.001	5	0	3	0	4
243	8	max	227.592	1	.352	4	.361	5	0	1	0	4	0	15
244		min	-312.37	3	.095	15	-.109	1	-.001	5	0	3	0	4
245	9	max	227.708	1	.306	4	.256	5	0	1	0	5	0	15
246		min	-312.283	3	.084	15	-.109	1	-.001	5	0	3	0	4
247	10	max	227.825	1	.26	4	.15	5	0	1	0	5	0	15
248		min	-312.195	3	.073	15	-.109	1	-.001	5	0	3	0	4
249	11	max	227.941	1	.215	4	.045	5	0	1	0	5	0	15
250		min	-312.108	3	.063	15	-.109	1	-.001	5	0	3	0	4
251	12	max	228.057	1	.169	4	-.004	10	0	1	0	5	0	15
252		min	-312.021	3	.049	12	-.109	1	-.001	5	0	3	0	4
253	13	max	228.174	1	.123	4	-.004	10	0	1	0	5	0	15
254		min	-311.934	3	.031	12	-.18	4	-.001	5	0	3	0	4
255	14	max	228.29	1	.078	4	-.004	10	0	1	0	5	0	15
256		min	-311.846	3	.014	12	-.285	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	228.407	1	.034	2	-.004	10	0	1	0	5	0	15
258		min	-311.759	3	-.007	3	-.391	4	-.001	5	0	3	0	4
259	16	max	228.523	1	.013	5	-.004	10	0	1	0	5	0	15
260		min	-311.672	3	-.034	3	-.496	4	-.001	5	0	3	0	4
261	17	max	228.639	1	-.002	15	-.004	10	0	1	0	5	0	15
262		min	-311.584	3	-.062	9	-.602	4	-.001	5	0	3	0	4
263	18	max	228.756	1	-.013	15	-.004	10	0	1	0	5	0	15
264		min	-311.497	3	-.106	6	-.707	4	-.001	5	0	3	0	4
265	19	max	228.872	1	-.023	15	-.004	10	0	1	0	5	0	15
266		min	-311.41	3	-.152	6	-.813	4	-.001	5	0	3	0	4
267	M11	1	max	138.766	2	1.77	.187	1	0	4	0	5	0	6
268		min	-132.544	3	.412	15	-1.255	5	0	10	0	1	0	15
269	2	max	138.697	2	1.593	6	.187	1	0	4	0	5	0	2
270		min	-132.595	3	.37	15	-1.122	5	0	10	0	1	0	12
271	3	max	138.628	2	1.416	6	.187	1	0	4	0	3	0	2
272		min	-132.647	3	.329	15	-.988	5	0	10	0	1	0	3
273	4	max	138.56	2	1.238	6	.187	1	0	4	0	3	0	15
274		min	-132.698	3	.287	15	-.854	5	0	10	0	1	0	4
275	5	max	138.491	2	1.061	6	.187	1	0	4	0	3	0	15
276		min	-132.75	3	.245	15	-.721	5	0	10	0	1	0	4
277	6	max	138.423	2	.884	6	.187	1	0	4	0	3	0	15
278		min	-132.801	3	.204	15	-.587	5	0	10	0	1	0	4
279	7	max	138.354	2	.707	6	.187	1	0	4	0	3	0	15
280		min	-132.852	3	.162	15	-.453	5	0	10	0	4	0	4
281	8	max	138.285	2	.529	6	.187	1	0	4	0	3	0	15
282		min	-132.904	3	.12	15	-.32	5	0	10	0	4	-.001	4
283	9	max	138.217	2	.352	6	.187	1	0	4	0	3	0	15
284		min	-132.955	3	.079	15	-.186	5	0	10	0	4	-.001	4
285	10	max	138.148	2	.175	6	.187	1	0	4	0	3	0	15
286		min	-133.007	3	.037	15	-.053	5	0	10	0	4	-.001	4
287	11	max	138.08	2	.029	2	.187	1	0	4	0	3	0	15
288		min	-133.058	3	-.034	3	-.04	3	0	10	0	4	-.001	4
289	12	max	138.011	2	-.046	15	.257	4	0	4	0	3	0	15
290		min	-133.11	3	-.18	4	-.04	3	0	10	0	4	-.001	4
291	13	max	137.942	2	-.088	15	.391	4	0	4	0	3	0	15
292		min	-133.161	3	-.357	4	-.04	3	0	10	0	4	-.001	4
293	14	max	137.874	2	-.129	15	.524	4	0	4	0	3	0	15
294		min	-133.213	3	-.534	4	-.04	3	0	10	0	4	-.001	4
295	15	max	137.805	2	-.171	15	.658	4	0	4	0	3	0	15
296		min	-133.264	3	-.711	4	-.04	3	0	10	0	5	0	4
297	16	max	137.737	2	-.213	15	.791	4	0	4	0	3	0	15
298		min	-133.316	3	-.888	4	-.04	3	0	10	0	10	0	4
299	17	max	137.668	2	-.254	15	.925	4	0	4	0	3	0	15
300		min	-133.367	3	-1.066	4	-.04	3	0	10	0	10	0	4
301	18	max	137.599	2	-.296	15	1.059	4	0	4	0	4	0	15
302		min	-133.418	3	-1.243	4	-.04	3	0	10	0	10	0	4
303	19	max	137.531	2	-.338	15	1.192	4	0	4	0	4	0	1
304		min	-133.47	3	-1.42	4	-.04	3	0	10	0	10	0	1
305	M12	1	max	308.728	1	0	.913	1	0	1	0	4	0	1
306		min	-32.767	3	0	1	-15.426	5	0	1	0	3	0	1
307	2	max	308.793	1	0	1	.913	1	0	1	0	1	0	1
308		min	-32.719	3	0	1	-15.482	5	0	1	-.001	5	0	1
309	3	max	308.858	1	0	1	.913	1	0	1	0	1	0	1
310		min	-32.67	3	0	1	-15.538	5	0	1	-.003	5	0	1
311	4	max	308.923	1	0	1	.913	1	0	1	0	1	0	1
312		min	-32.622	3	0	1	-15.594	5	0	1	-.004	5	0	1
313	5	max	308.987	1	0	1	.913	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-32.573	3	0	1	-15.65	5	0	1	-.006	5	0	1
315	6	max	309.052	1	0	1	.913	1	0	1	0	1	0	1
316		min	-32.524	3	0	1	-15.706	5	0	1	-.007	5	0	1
317	7	max	309.117	1	0	1	.913	1	0	1	0	1	0	1
318		min	-32.476	3	0	1	-15.762	5	0	1	-.008	5	0	1
319	8	max	309.181	1	0	1	.913	1	0	1	0	1	0	1
320		min	-32.427	3	0	1	-15.818	5	0	1	-.01	5	0	1
321	9	max	309.246	1	0	1	.913	1	0	1	0	1	0	1
322		min	-32.379	3	0	1	-15.874	5	0	1	-.011	5	0	1
323	10	max	309.311	1	0	1	.913	1	0	1	0	1	0	1
324		min	-32.33	3	0	1	-15.931	5	0	1	-.013	5	0	1
325	11	max	309.376	1	0	1	.913	1	0	1	0	1	0	1
326		min	-32.282	3	0	1	-15.987	5	0	1	-.014	5	0	1
327	12	max	309.44	1	0	1	.913	1	0	1	0	1	0	1
328		min	-32.233	3	0	1	-16.043	5	0	1	-.015	5	0	1
329	13	max	309.505	1	0	1	.913	1	0	1	0	1	0	1
330		min	-32.185	3	0	1	-16.099	5	0	1	-.017	5	0	1
331	14	max	309.57	1	0	1	.913	1	0	1	.001	1	0	1
332		min	-32.136	3	0	1	-16.155	5	0	1	-.018	5	0	1
333	15	max	309.634	1	0	1	.913	1	0	1	.001	1	0	1
334		min	-32.088	3	0	1	-16.211	5	0	1	-.02	5	0	1
335	16	max	309.699	1	0	1	.913	1	0	1	.001	1	0	1
336		min	-32.039	3	0	1	-16.267	5	0	1	-.021	5	0	1
337	17	max	309.764	1	0	1	.913	1	0	1	.001	1	0	1
338		min	-31.991	3	0	1	-16.323	5	0	1	-.023	5	0	1
339	18	max	309.829	1	0	1	.913	1	0	1	.001	1	0	1
340		min	-31.942	3	0	1	-16.379	5	0	1	-.024	5	0	1
341	19	max	309.893	1	0	1	.913	1	0	1	.001	1	0	1
342		min	-31.894	3	0	1	-16.435	5	0	1	-.026	5	0	1
343	M1	1	max	76.594	1	345.479	3	-.189	10	0	.04	1	0	2
344			min	5.84	10	-232.724	2	-20.15	1	0	3	0	10	3
345	2	max	76.712	1	345.29	3	-.189	10	0	2	.035	1	.051	2
346		min	5.907	12	-232.977	2	-20.15	1	0	3	0	10	-.075	3
347	3	max	60.729	3	4.78	14	-.186	10	0	5	.031	1	.1	2
348		min	-8.842	10	-19.894	2	-20.063	1	0	1	0	10	-.149	3
349	4	max	60.818	3	4.532	14	-.186	10	0	5	.026	1	.105	2
350		min	-8.744	10	-20.147	2	-20.063	1	0	1	0	10	-.145	3
351	5	max	60.906	3	4.283	14	-.186	10	0	5	.022	1	.109	2
352		min	-8.646	10	-20.4	2	-20.063	1	0	1	0	10	-.141	3
353	6	max	60.995	3	4.034	14	-.186	10	0	5	.018	1	.114	2
354		min	-8.547	10	-20.653	2	-20.063	1	0	1	0	10	-.137	3
355	7	max	61.083	3	3.786	14	-.186	10	0	5	.013	1	.118	2
356		min	-8.449	10	-20.906	2	-20.063	1	0	1	0	10	-.133	3
357	8	max	61.172	3	3.537	14	-.186	10	0	5	.009	1	.123	2
358		min	-8.351	10	-21.159	2	-20.063	1	0	1	0	10	-.129	3
359	9	max	61.26	3	3.288	14	-.186	10	0	5	.004	1	.127	2
360		min	-8.252	10	-21.412	2	-20.063	1	0	1	0	10	-.125	3
361	10	max	61.349	3	3.04	14	-.186	10	0	5	.002	3	.132	2
362		min	-8.154	10	-21.665	2	-20.063	1	0	1	0	10	-.121	3
363	11	max	61.437	3	2.791	14	-.186	10	0	5	0	3	.137	2
364		min	-8.056	10	-21.918	2	-20.063	1	0	1	-.004	1	-.117	3
365	12	max	61.526	3	2.572	9	-.186	10	0	5	0	10	.142	2
366		min	-7.957	10	-22.171	2	-20.063	1	0	1	-.009	1	-.112	3
367	13	max	61.614	3	2.361	9	-.186	10	0	5	0	10	.146	2
368		min	-7.859	10	-22.425	2	-20.063	1	0	1	-.013	1	-.108	3
369	14	max	61.703	3	2.15	9	-.186	10	0	5	0	10	.151	2
370		min	-7.761	10	-22.678	2	-20.063	1	0	1	-.017	1	-.104	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	61.791	3	1.939	9	-1.186	10	0	5	0	10	.156	2
372			min	-7.662	10	-22.931	2	-20.063	1	0	1	-.022	1	-.1	3
373		16	max	86.69	2	84.298	2	-1.188	10	0	1	0	10	.16	2
374			min	-20.253	3	-124.414	3	-20.22	1	0	5	-.026	1	-.094	3
375		17	max	86.808	2	84.045	2	-1.188	10	0	1	0	10	.142	2
376			min	-20.164	3	-124.604	3	-20.22	1	0	5	-.031	1	-.067	3
377		18	max	-3.894	12	328.871	2	-1.187	10	0	3	0	10	.072	2
378			min	-76.697	1	-155.01	3	-27.601	4	0	2	-.035	1	-.034	3
379		19	max	-3.835	12	328.618	2	-1.187	10	0	3	0	10	0	2
380			min	-76.579	1	-155.2	3	-27.359	4	0	2	-.04	1	0	3
381	M5	1	max	185.084	1	1113.127	3	0	2	0	9	.031	4	0	3
382			min	.045	3	-743.315	2	-66.769	3	0	5	0	10	0	2
383		2	max	185.202	1	1112.937	3	0	2	0	9	.026	4	.161	2
384			min	.133	3	-743.568	2	-66.769	3	0	5	-.005	3	-.241	3
385		3	max	165.703	3	5.401	9	7.271	3	0	3	.022	4	.319	2
386			min	-25.064	10	-69.435	2	-16.979	4	0	4	-.019	3	-.477	3
387		4	max	165.791	3	5.191	9	7.271	3	0	3	.018	4	.334	2
388			min	-24.965	10	-69.689	2	-16.737	4	0	4	-.017	3	-.463	3
389		5	max	165.88	3	4.98	9	7.271	3	0	3	.015	4	.35	2
390			min	-24.867	10	-69.942	2	-16.495	4	0	4	-.016	3	-.449	3
391		6	max	165.968	3	4.769	9	7.271	3	0	3	.011	4	.365	2
392			min	-24.769	10	-70.195	2	-16.253	4	0	4	-.014	3	-.435	3
393		7	max	166.057	3	4.558	9	7.271	3	0	3	.008	4	.38	2
394			min	-24.67	10	-70.448	2	-16.011	4	0	4	-.013	3	-.421	3
395		8	max	166.145	3	4.347	9	7.271	3	0	3	.004	4	.395	2
396			min	-24.572	10	-70.701	2	-15.769	4	0	4	-.011	3	-.406	3
397		9	max	166.234	3	4.136	9	7.271	3	0	3	0	4	.411	2
398			min	-24.474	10	-70.954	2	-15.527	4	0	4	-.009	3	-.392	3
399		10	max	166.322	3	3.925	9	7.271	3	0	3	0	2	.426	2
400			min	-24.375	10	-71.207	2	-15.285	4	0	4	-.008	3	-.378	3
401		11	max	166.411	3	3.714	9	7.271	3	0	3	0	2	.442	2
402			min	-24.277	10	-71.46	2	-15.043	4	0	4	-.006	3	-.364	3
403		12	max	166.499	3	3.503	9	7.271	3	0	3	0	2	.457	2
404			min	-24.179	10	-71.713	2	-14.801	4	0	4	-.009	4	-.349	3
405		13	max	166.588	3	3.292	9	7.271	3	0	3	0	2	.473	2
406			min	-24.08	10	-71.966	2	-14.559	4	0	4	-.012	4	-.335	3
407		14	max	166.676	3	3.082	9	7.271	3	0	3	0	2	.488	2
408			min	-23.982	10	-72.219	2	-14.317	4	0	4	-.015	4	-.32	3
409		15	max	166.765	3	2.871	9	7.271	3	0	3	0	3	.504	2
410			min	-23.884	10	-72.472	2	-14.075	4	0	4	-.019	4	-.306	3
411		16	max	277.785	2	294.666	2	7.238	3	0	3	.001	3	.517	2
412			min	-65.141	3	-358.678	3	-12.788	4	0	4	-.022	4	-.289	3
413		17	max	277.903	2	294.413	2	7.238	3	0	3	.003	3	.453	2
414			min	-65.053	3	-358.868	3	-12.546	4	0	4	-.024	4	-.211	3
415		18	max	-3.679	12	1051.979	2	6.667	3	0	4	.004	3	.227	2
416			min	-185.237	1	-488.406	3	-28.75	5	0	9	-.031	4	-.106	3
417		19	max	-3.62	12	1051.726	2	6.667	3	0	4	.006	3	0	3
418			min	-185.119	1	-488.595	3	-28.508	5	0	9	-.037	4	0	2
419	M9	1	max	76.457	1	345.41	3	120.648	4	0	3	0	15	0	2
420			min	.816	15	-232.724	2	.189	10	0	2	-.039	1	0	3
421		2	max	76.575	1	345.22	3	120.89	4	0	3	.025	5	.051	2
422			min	.851	15	-232.977	2	.189	10	0	2	-.035	1	-.075	3
423		3	max	60.35	3	4.455	9	19.769	1	0	1	.049	5	.1	2
424			min	-8.499	10	-19.869	2	-21.93	5	0	5	-.03	1	-.148	3
425		4	max	60.438	3	4.244	9	19.769	1	0	1	.044	5	.105	2
426			min	-8.401	10	-20.122	2	-21.688	5	0	5	-.026	1	-.145	3
427		5	max	60.527	3	4.033	9	19.769	1	0	1	.039	5	.109	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-8.302	10	-20.375	2	-21.446	5	0	5	-.022	1	-.141	3
429	6	max	60.615	3	3.822	9	19.769	1	0	1	.034	5	.114	2
430		min	-8.204	10	-20.628	2	-21.204	5	0	5	-.017	1	-.137	3
431	7	max	60.704	3	3.611	9	19.769	1	0	1	.03	5	.118	2
432		min	-8.106	10	-20.881	2	-20.962	5	0	5	-.013	1	-.133	3
433	8	max	60.792	3	3.4	9	19.769	1	0	1	.025	5	.123	2
434		min	-8.007	10	-21.134	2	-20.72	5	0	5	-.009	1	-.129	3
435	9	max	60.881	3	3.189	9	19.769	1	0	1	.021	5	.127	2
436		min	-7.909	10	-21.387	2	-20.478	5	0	5	-.004	1	-.125	3
437	10	max	60.969	3	2.978	9	19.769	1	0	1	.017	4	.132	2
438		min	-7.811	10	-21.64	2	-20.236	5	0	5	0	1	-.121	3
439	11	max	61.058	3	2.768	9	19.769	1	0	1	.013	4	.137	2
440		min	-7.712	10	-21.893	2	-19.994	5	0	5	0	10	-.117	3
441	12	max	61.146	3	2.557	9	19.769	1	0	1	.01	4	.141	2
442		min	-7.614	10	-22.146	2	-19.752	5	0	5	0	10	-.112	3
443	13	max	61.235	3	2.346	9	19.769	1	0	1	.013	1	.146	2
444		min	-7.516	10	-22.399	2	-19.51	5	0	5	0	10	-.108	3
445	14	max	61.323	3	2.135	9	19.769	1	0	1	.017	1	.151	2
446		min	-7.417	10	-22.652	2	-19.268	5	0	5	0	5	-.104	3
447	15	max	61.412	3	1.924	9	19.769	1	0	1	.021	1	.156	2
448		min	-7.319	10	-22.905	2	-19.026	5	0	5	-.005	5	-.1	3
449	16	max	86.835	2	83.989	2	19.937	1	0	10	.026	1	.16	2
450		min	-21.074	3	-124.886	3	-17.635	5	0	4	-.008	5	-.094	3
451	17	max	86.953	2	83.736	2	19.937	1	0	10	.03	1	.142	2
452		min	-20.986	3	-125.076	3	-17.393	5	0	4	-.012	5	-.067	3
453	18	max	6.963	5	328.871	2	20.852	1	0	2	.035	1	.072	2
454		min	-76.555	1	-155.002	3	-32.567	5	0	3	-.019	5	-.034	3
455	19	max	7.018	5	328.618	2	20.852	1	0	2	.039	1	0	2
456		min	-76.437	1	-155.192	3	-32.325	5	0	3	-.026	5	0	3
457	M13	1	max	120.648	4	232.638	2	-.816	15	0	.039	1	0	2
458		min	.189	10	-345.449	3	-76.452	1	0	3	0	15	0	3
459	2	max	116.01	4	165.54	2	-.133	15	0	2	.012	3	.139	3
460		min	.189	10	-245.229	3	-57.667	1	0	3	-.002	10	-.094	2
461	3	max	111.372	4	98.442	2	.73	5	0	2	.009	3	.232	3
462		min	.189	10	-145.01	3	-38.881	1	0	3	-.015	1	-.156	2
463	4	max	106.734	4	31.345	2	1.786	5	0	2	.006	3	.276	3
464		min	.189	10	-44.791	3	-20.096	1	0	3	-.029	1	-.187	2
465	5	max	102.096	4	55.429	3	2.841	5	0	2	.004	3	.274	3
466		min	.189	10	-35.753	2	-4.593	3	0	3	-.034	1	-.186	2
467	6	max	97.458	4	155.648	3	17.475	1	0	2	.003	5	.224	3
468		min	.189	10	-102.851	2	-3.6	3	0	3	-.03	1	-.153	2
469	7	max	92.82	4	255.867	3	36.26	1	0	2	.005	5	.127	3
470		min	.189	10	-169.949	2	-2.607	3	0	3	-.018	1	-.089	2
471	8	max	88.182	4	356.087	3	55.045	1	0	2	.008	4	.009	1
472		min	.189	10	-237.047	2	-1.614	3	0	3	0	3	-.018	3
473	9	max	83.544	4	456.306	3	73.831	1	0	2	.034	1	.135	2
474		min	.189	10	-304.144	2	-.621	3	0	3	-.001	3	-.209	3
475	10	max	78.906	4	556.526	3	92.616	1	0	2	.074	1	.295	2
476		min	.189	10	-371.242	2	.372	3	0	3	-.016	5	-.449	3
477	11	max	54.604	4	304.144	2	5.277	5	0	3	.034	1	.135	2
478		min	.189	10	-456.306	3	-73.693	1	0	2	-.013	5	-.209	3
479	12	max	49.966	4	237.047	2	6.333	5	0	3	.005	2	.009	1
480		min	.189	10	-356.087	3	-54.908	1	0	2	-.011	5	-.018	3
481	13	max	45.328	4	169.949	2	7.388	5	0	3	0	10	.127	3
482		min	.189	10	-255.867	3	-36.123	1	0	2	-.018	1	-.089	2
483	14	max	40.69	4	102.851	2	8.444	5	0	3	-.002	10	.224	3
484		min	.189	10	-155.648	3	-17.337	1	0	2	-.03	1	-.153	2



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	36.052	4	35.753	2	10.228	4	0	3	0	5	.274	3
486			min	.189	10	-55.429	3	-2.144	2	0	2	-.034	1	-.186	2
487		16	max	31.414	4	44.791	3	20.233	1	0	3	.005	5	.276	3
488			min	.189	10	-31.345	2	.186	10	0	2	-.029	1	-.187	2
489		17	max	26.776	4	145.01	3	39.019	1	0	3	.011	5	.232	3
490			min	.189	10	-98.442	2	2.071	10	0	2	-.015	1	-.156	2
491		18	max	22.138	4	245.229	3	57.804	1	0	3	.019	4	.139	3
492			min	.189	10	-165.54	2	3.955	10	0	2	-.002	10	-.094	2
493		19	max	20.187	1	345.449	3	76.589	1	0	3	.04	1	0	2
494			min	.189	10	-232.638	2	5.84	10	0	2	0	10	0	3
495	M16	1	max	32.315	5	328.723	2	7.018	5	0	3	.039	1	0	2
496			min	-20.815	1	-155.214	3	-76.442	1	0	2	-.026	5	0	3
497		2	max	27.677	5	233.712	2	8.074	5	0	3	.008	1	.063	3
498			min	-20.815	1	-110.821	3	-57.657	1	0	2	-.022	5	-.133	2
499		3	max	23.039	5	138.701	2	9.129	5	0	3	0	3	.105	3
500			min	-20.815	1	-66.428	3	-38.871	1	0	2	-.021	4	-.221	2
501		4	max	18.401	5	43.69	2	10.185	5	0	3	-.001	12	.126	3
502			min	-20.815	1	-22.035	3	-20.086	1	0	2	-.029	1	-.264	2
503		5	max	13.763	5	22.357	3	11.24	5	0	3	-.002	12	.125	3
504			min	-20.815	1	-51.321	2	-3.002	3	0	2	-.034	1	-.262	2
505		6	max	9.125	5	66.75	3	17.485	1	0	3	-.002	15	.104	3
506			min	-20.815	1	-146.331	2	-2.009	3	0	2	-.03	1	-.215	2
507		7	max	4.487	5	111.143	3	36.27	1	0	3	.003	5	.062	3
508			min	-20.815	1	-241.342	2	-1.016	3	0	2	-.018	1	-.124	2
509		8	max	2.14	3	155.536	3	55.055	1	0	3	.01	4	.013	2
510			min	-20.815	1	-336.353	2	-.023	3	0	2	-.006	3	0	3
511		9	max	2.14	3	199.928	3	73.841	1	0	3	.034	1	.194	2
512			min	-20.815	1	-431.364	2	.836	12	0	2	-.006	3	-.084	3
513		10	max	19.089	5	-8.093	15	92.626	1	0	14	.074	1	.42	2
514			min	-20.815	1	-526.375	2	-3.009	3	0	2	-.005	3	-.189	3
515		11	max	14.451	5	431.364	2	4.472	5	0	2	.034	1	.194	2
516			min	-20.769	1	-199.928	3	-73.698	1	0	3	-.01	5	-.084	3
517		12	max	9.813	5	336.353	2	5.527	5	0	2	.005	2	.013	2
518			min	-20.769	1	-155.536	3	-54.913	1	0	3	-.008	5	0	3
519		13	max	5.175	5	241.342	2	6.582	5	0	2	0	10	.062	3
520			min	-20.769	1	-111.143	3	-36.128	1	0	3	-.018	1	-.124	2
521		14	max	.537	5	146.331	2	7.638	5	0	2	0	12	.104	3
522			min	-20.769	1	-66.75	3	-17.342	1	0	3	-.03	1	-.215	2
523		15	max	-.187	10	51.321	2	9.4	4	0	2	.002	5	.125	3
524			min	-20.769	1	-22.357	3	-2.134	2	0	3	-.034	1	-.262	2
525		16	max	-.187	10	22.035	3	20.228	1	0	2	.006	5	.126	3
526			min	-20.769	1	-43.69	2	.191	10	0	3	-.029	1	-.264	2
527		17	max	-.187	10	66.428	3	39.014	1	0	2	.011	5	.105	3
528			min	-20.769	1	-138.701	2	2.075	10	0	3	-.015	1	-.221	2
529		18	max	-.187	10	110.821	3	57.799	1	0	2	.019	4	.063	3
530			min	-22.745	4	-233.712	2	3.173	12	0	3	-.002	10	-.133	2
531		19	max	-.187	10	155.214	3	76.584	1	0	2	.04	1	0	2
532			min	-27.383	4	-328.723	2	3.835	12	0	3	0	10	0	3
533	M15	1	max	0	1	.879	3	.124	3	0	1	0	1	0	1
534			min	-90.83	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.781	3	.124	3	0	1	0	1	0	1
536			min	-90.895	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.684	3	.124	3	0	1	0	1	0	1
538			min	-90.961	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.586	3	.124	3	0	1	0	1	0	1
540			min	-91.026	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.488	3	.124	3	0	1	0	1	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-91.091	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.391	3	.124	3	0	1	0	1	0	1
544			min	-91.156	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.293	3	.124	3	0	1	0	3	0	1
546			min	-91.221	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.195	3	.124	3	0	1	0	3	0	1
548			min	-91.287	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.098	3	.124	3	0	1	0	3	0	1
550			min	-91.352	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.124	3	0	1	0	3	0	1
552			min	-91.417	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.124	3	0	1	0	3	0	1
554			min	-91.482	3	-.098	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.124	3	0	1	0	3	0	1
556			min	-91.547	3	-.195	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.124	3	0	1	0	3	0	1
558			min	-91.613	3	-.293	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.124	3	0	1	0	3	0	1
560			min	-91.678	3	-.391	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.124	3	0	1	0	3	0	1
562			min	-91.743	3	-.488	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.124	3	0	1	0	3	0	1
564			min	-91.808	3	-.586	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.124	3	0	1	0	3	0	1
566			min	-91.873	3	-.684	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.124	3	0	1	0	3	0	1
568			min	-91.938	3	-.781	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.124	3	0	1	0	3	0	1
570			min	-92.004	3	-.879	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.168	4	.283	4	0	3	0	3	0	1
572			min	-173.207	4	0	2	-.05	3	0	1	0	4	0	1
573		2	max	0	2	1.927	4	.255	4	0	3	0	3	0	2
574			min	-173.206	4	0	2	-.05	3	0	1	0	4	0	4
575		3	max	0	2	1.686	4	.227	4	0	3	0	3	0	2
576			min	-173.205	4	0	2	-.05	3	0	1	0	4	-.001	4
577		4	max	0	2	1.445	4	.2	4	0	3	0	3	0	2
578			min	-173.204	4	0	2	-.05	3	0	1	0	1	-.002	4
579		5	max	0	2	1.204	4	.172	4	0	3	0	3	0	2
580			min	-173.203	4	0	2	-.05	3	0	1	0	1	-.002	4
581		6	max	0	2	.964	4	.144	4	0	3	0	3	0	2
582			min	-173.202	4	0	2	-.05	3	0	1	0	1	-.002	4
583		7	max	0	2	.723	4	.117	4	0	3	0	3	0	2
584			min	-173.201	4	0	2	-.05	3	0	1	0	1	-.003	4
585		8	max	0	2	.482	4	.089	4	0	3	0	5	0	2
586			min	-173.2	4	0	2	-.05	3	0	1	0	1	-.003	4
587		9	max	0	2	.241	4	.061	4	0	3	0	5	0	2
588			min	-173.199	4	0	2	-.05	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.039	1	0	3	0	5	0	2
590			min	-173.198	4	0	1	-.05	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.039	1	0	3	0	5	0	2
592			min	-173.197	4	-.241	4	-.05	3	0	1	0	1	-.003	4
593		12	max	.013	11	0	2	.039	1	0	3	0	5	0	2
594			min	-173.196	4	-.482	4	-.05	3	0	1	0	1	-.003	4
595		13	max	.085	11	0	2	.039	1	0	3	0	5	0	2
596			min	-173.195	4	-.723	4	-.053	5	0	1	0	3	-.003	4
597		14	max	.158	11	0	2	.039	1	0	3	0	5	0	2
598			min	-173.194	4	-.964	4	-.081	5	0	1	0	3	-.002	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.23	11	0	2	.039	1	0	3	0	5	0	2
600		min	-173.193	4	-1.204	4	-.108	5	0	1	0	3	-.002	4
601	16	max	.302	11	0	2	.039	1	0	3	0	4	0	2
602		min	-173.191	4	-1.445	4	-.136	5	0	1	0	3	-.002	4
603	17	max	.375	11	0	2	.039	1	0	3	0	1	0	2
604		min	-173.19	4	-1.686	4	-.164	5	0	1	0	3	-.001	4
605	18	max	.447	11	0	2	.039	1	0	3	0	1	0	2
606		min	-173.189	4	-1.927	4	-.191	5	0	1	0	3	0	4
607	19	max	.52	11	0	2	.039	1	0	3	0	1	0	1
608		min	-173.232	5	-2.168	4	-.219	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.008	2	.003	1	9.535e-4	5	NC	3	NC	1	
2			min	-.003	3	-.008	3	-.01	5	-3.111e-4	1	4524.743	2	NC	1	
3			2	max	.002	1	.007	2	.003	1	9.739e-4	5	NC	3	NC	1
4				min	-.003	3	-.007	3	-.009	5	-2.975e-4	1	4929.118	2	NC	1
5			3	max	.002	1	.007	2	.003	1	9.942e-4	5	NC	3	NC	1
6				min	-.003	3	-.007	3	-.009	5	-2.839e-4	1	5408.483	2	NC	1
7			4	max	.002	1	.006	2	.003	1	1.015e-3	5	NC	1	NC	1
8				min	-.003	3	-.007	3	-.009	5	-2.703e-4	1	5980.745	2	NC	1
9			5	max	.002	1	.005	2	.002	1	1.035e-3	5	NC	1	NC	1
10				min	-.003	3	-.006	3	-.009	5	-2.566e-4	1	6669.722	2	NC	1
11			6	max	.001	1	.005	2	.002	1	1.055e-3	5	NC	1	NC	1
12				min	-.002	3	-.006	3	-.008	5	-2.43e-4	1	7507.614	2	NC	1
13			7	max	.001	1	.004	2	.002	1	1.076e-3	5	NC	1	NC	1
14				min	-.002	3	-.006	3	-.008	5	-2.294e-4	1	8538.79	2	NC	1
15			8	max	.001	1	.004	2	.002	1	1.096e-3	5	NC	1	NC	1
16				min	-.002	3	-.005	3	-.007	5	-2.158e-4	1	9825.769	2	NC	1
17			9	max	.001	1	.003	2	.001	1	1.116e-3	5	NC	1	NC	1
18				min	-.002	3	-.005	3	-.007	5	-2.021e-4	1	NC	1	NC	1
19			10	max	.001	1	.003	2	.001	1	1.137e-3	5	NC	1	NC	1
20				min	-.002	3	-.005	3	-.006	5	-1.885e-4	1	NC	1	NC	1
21		11	max	0	1	.002	2	.001	1	1.157e-3	5	NC	1	NC	1	
22			min	-.001	3	-.004	3	-.006	5	-1.749e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	0	1	1.177e-3	5	NC	1	NC	1	
24			min	-.001	3	-.004	3	-.005	5	-1.613e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	1.198e-3	5	NC	1	NC	1	
26			min	-.001	3	-.003	3	-.005	5	-1.477e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.218e-3	5	NC	1	NC	1	
28			min	0	3	-.003	3	-.004	5	-1.34e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.239e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.003	5	-1.204e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.259e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-1.068e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.279e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-9.316e-5	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.3e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-7.954e-5	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.32e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-6.592e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	3.073e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	-6.14e-4	5	NC	1	NC	1	
41			2	max	0	3	0	2	.003	5	4.028e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	-6.179e-4	5	NC	1	NC	1



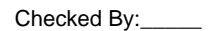
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.006	5	4.982e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.218e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.01	5	5.937e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.257e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.013	5	6.891e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.296e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.016	4	7.846e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-6.335e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.019	4	8.8e-5	1	NC	1	NC	1
52			min	0	2	-.005	3	0	9	-6.374e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.022	4	9.755e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-6.413e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.025	4	1.071e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	10	-6.452e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.028	4	1.166e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-6.491e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.031	4	1.262e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-6.53e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.033	4	1.357e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-6.569e-4	5	NC	1	NC	1
63		13	max	.001	3	.003	2	.036	4	1.453e-4	1	NC	1	NC	1
64			min	-.001	2	-.007	3	0	10	-6.608e-4	5	NC	1	NC	1
65		14	max	.001	3	.004	2	.039	4	1.548e-4	1	NC	1	NC	1
66			min	-.001	2	-.007	3	0	10	-6.647e-4	5	NC	1	NC	1
67		15	max	.001	3	.005	2	.041	4	1.644e-4	1	NC	1	NC	1
68			min	-.001	2	-.008	3	0	10	-6.686e-4	5	9674.547	2	NC	1
69		16	max	.001	3	.006	2	.043	4	1.739e-4	1	NC	1	NC	1
70			min	-.001	2	-.008	3	0	10	-6.725e-4	5	8156.831	2	NC	1
71		17	max	.001	3	.007	2	.046	4	1.835e-4	1	NC	1	NC	1
72			min	-.001	2	-.008	3	0	10	-6.764e-4	5	6990.62	2	NC	1
73		18	max	.001	3	.008	2	.048	4	1.93e-4	1	NC	3	NC	1
74			min	-.001	2	-.008	3	0	10	-6.803e-4	5	6083.569	2	NC	1
75		19	max	.002	3	.009	2	.05	4	2.026e-4	1	NC	3	NC	1
76			min	-.002	2	-.008	3	0	10	-6.842e-4	5	5371.337	2	NC	1
77	M4	1	max	.001	1	.009	2	0	10	2.991e-3	5	NC	1	NC	1
78			min	0	3	-.008	3	-.053	4	-2.478e-4	1	NC	1	362.753	4
79		2	max	.001	1	.009	2	0	10	2.991e-3	5	NC	1	NC	1
80			min	0	3	-.007	3	-.049	4	-2.478e-4	1	NC	1	395.41	4
81		3	max	.001	1	.008	2	0	10	2.991e-3	5	NC	1	NC	1
82			min	0	3	-.007	3	-.045	4	-2.478e-4	1	NC	1	434.274	4
83		4	max	.001	1	.008	2	0	10	2.991e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.04	4	-2.478e-4	1	NC	1	480.98	4
85		5	max	.001	1	.007	2	0	10	2.991e-3	5	NC	1	NC	1
86			min	0	3	-.006	3	-.036	4	-2.478e-4	1	NC	1	537.754	4
87		6	max	.001	1	.007	2	0	10	2.991e-3	5	NC	1	NC	1
88			min	0	3	-.006	3	-.032	4	-2.478e-4	1	NC	1	607.693	4
89		7	max	0	1	.006	2	0	10	2.991e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.028	4	-2.478e-4	1	NC	1	695.206	4
91		8	max	0	1	.006	2	0	10	2.991e-3	5	NC	1	NC	1
92			min	0	3	-.005	3	-.024	4	-2.478e-4	1	NC	1	806.75	4
93		9	max	0	1	.005	2	0	10	2.991e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.02	4	-2.478e-4	1	NC	1	952.091	4
95		10	max	0	1	.005	2	0	10	2.991e-3	5	NC	1	NC	1
96			min	0	3	-.004	3	-.017	4	-2.478e-4	1	NC	1	1146.579	4
97		11	max	0	1	.004	2	0	10	2.991e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.014	4	-2.478e-4	1	NC	1	1415.494	4
99		12	max	0	1	.004	2	0	10	2.991e-3	5	NC	1	NC	1



RISA-3D Version 13.0.0 \.....\PVMini 60 Cell 1V 25° 150mph 30psf 4.25ft 7-10Pa Page 34



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.007	4	1.234e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-6.259e-4	4	NC	1	NC	1
159		4	max	0	3	.004	2	.01	4	1.094e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-6.186e-4	4	NC	1	NC	1
161		5	max	0	3	.005	2	.013	4	9.526e-6	1	NC	1	NC	1
162			min	-.001	2	-.007	3	0	1	-6.114e-4	4	9270.493	2	NC	1
163		6	max	.001	3	.006	2	.017	4	1.938e-5	3	NC	1	NC	1
164			min	-.001	2	-.009	3	0	1	-6.041e-4	4	7431.354	2	NC	1
165		7	max	.001	3	.007	2	.02	4	4.159e-5	3	NC	1	NC	1
166			min	-.002	2	-.01	3	0	1	-5.969e-4	4	6171.395	2	NC	1
167		8	max	.002	3	.009	2	.023	4	6.38e-5	3	NC	3	NC	1
168			min	-.002	2	-.012	3	0	1	-5.896e-4	4	5246.116	2	NC	1
169		9	max	.002	3	.01	2	.026	4	8.6e-5	3	NC	3	NC	1
170			min	-.002	2	-.013	3	0	1	-5.824e-4	4	4533.765	2	NC	1
171		10	max	.002	3	.012	2	.029	4	1.082e-4	3	NC	3	NC	1
172			min	-.003	2	-.015	3	0	1	-5.751e-4	4	3966.879	2	NC	1
173		11	max	.002	3	.013	2	.032	4	1.304e-4	3	NC	3	NC	1
174			min	-.003	2	-.016	3	0	1	-5.679e-4	4	3504.933	2	NC	1
175		12	max	.003	3	.015	2	.035	4	1.526e-4	3	NC	3	NC	1
176			min	-.003	2	-.017	3	0	1	-5.606e-4	4	3121.964	2	NC	1
177		13	max	.003	3	.016	2	.037	4	1.748e-4	3	NC	3	NC	1
178			min	-.004	2	-.018	3	0	1	-5.534e-4	4	2800.42	2	NC	1
179		14	max	.003	3	.018	2	.04	4	1.97e-4	3	NC	3	NC	1
180			min	-.004	2	-.019	3	0	1	-5.461e-4	4	2527.895	2	NC	1
181		15	max	.003	3	.02	2	.042	4	2.192e-4	3	NC	3	NC	1
182			min	-.004	2	-.02	3	0	1	-5.389e-4	4	2295.28	2	NC	1
183		16	max	.004	3	.022	2	.045	4	2.414e-4	3	NC	3	NC	1
184			min	-.004	2	-.021	3	0	1	-5.316e-4	4	2095.685	2	NC	1
185		17	max	.004	3	.024	2	.047	4	2.637e-4	3	NC	3	NC	1
186			min	-.005	2	-.022	3	0	9	-5.244e-4	4	1923.765	2	NC	1
187		18	max	.004	3	.026	2	.049	4	2.859e-4	3	NC	3	NC	1
188			min	-.005	2	-.023	3	0	9	-5.171e-4	4	1775.297	2	NC	1
189		19	max	.004	3	.028	2	.052	4	3.081e-4	3	NC	3	NC	1
190			min	-.005	2	-.024	3	0	9	-5.099e-4	4	1646.898	2	NC	1
191	M8	1	max	.004	1	.03	2	0	9	2.829e-3	4	NC	1	NC	1
192			min	0	3	-.024	3	-.054	4	-2.313e-4	3	NC	1	356.563	4
193		2	max	.004	1	.028	2	0	9	2.829e-3	4	NC	1	NC	1
194			min	0	3	-.022	3	-.05	4	-2.313e-4	3	NC	1	388.664	4
195		3	max	.004	1	.026	2	0	9	2.829e-3	4	NC	1	NC	1
196			min	0	3	-.021	3	-.045	4	-2.313e-4	3	NC	1	426.866	4
197		4	max	.003	1	.025	2	0	9	2.829e-3	4	NC	1	NC	1
198			min	0	3	-.02	3	-.041	4	-2.313e-4	3	NC	1	472.776	4
199		5	max	.003	1	.023	2	0	9	2.829e-3	4	NC	1	NC	1
200			min	0	3	-.018	3	-.037	4	-2.313e-4	3	NC	1	528.584	4
201		6	max	.003	1	.021	2	0	9	2.829e-3	4	NC	1	NC	1
202			min	0	3	-.017	3	-.032	4	-2.313e-4	3	NC	1	597.333	4
203		7	max	.003	1	.02	2	0	9	2.829e-3	4	NC	1	NC	1
204			min	0	3	-.016	3	-.028	4	-2.313e-4	3	NC	1	683.357	4
205		8	max	.003	1	.018	2	0	9	2.829e-3	4	NC	1	NC	1
206			min	0	3	-.014	3	-.024	4	-2.313e-4	3	NC	1	793.004	4
207		9	max	.002	1	.017	2	0	9	2.829e-3	4	NC	1	NC	1
208			min	0	3	-.013	3	-.021	4	-2.313e-4	3	NC	1	935.874	4
209		10	max	.002	1	.015	2	0	9	2.829e-3	4	NC	1	NC	1
210			min	0	3	-.012	3	-.017	4	-2.313e-4	3	NC	1	1127.056	4
211		11	max	.002	1	.013	2	0	9	2.829e-3	4	NC	1	NC	1
212			min	0	3	-.01	3	-.014	4	-2.313e-4	3	NC	1	1391.401	4
213		12	max	.002	1	.012	2	0	9	2.829e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.005	4	5.272e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-5.926e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.008	4	3.077e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-6.387e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.011	4	8.819e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-6.847e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.013	4	-7.827e-7	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-7.307e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.016	5	-8.912e-7	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-7.768e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.019	5	-9.998e-7	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-8.228e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.021	5	-1.108e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.003	3	-8.688e-4	4	NC	1	NC	1
285		10	max	0	3	.002	2	.024	5	-1.217e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-9.148e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.026	5	-1.325e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.003	3	-9.609e-4	4	NC	1	NC	1
289		12	max	0	3	.003	2	.029	5	-1.434e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-1.007e-3	4	NC	1	NC	1
291		13	max	.001	3	.003	2	.031	5	-1.542e-6	10	NC	1	NC	1
292			min	-.001	2	-.007	3	-.003	3	-1.053e-3	4	NC	1	NC	1
293		14	max	.001	3	.004	2	.034	5	-1.651e-6	10	NC	1	NC	1
294			min	-.001	2	-.008	3	-.003	3	-1.099e-3	4	NC	1	NC	1
295		15	max	.001	3	.005	2	.036	5	-1.76e-6	10	NC	1	NC	1
296			min	-.001	2	-.008	3	-.003	3	-1.145e-3	4	9688.504	2	NC	1
297		16	max	.001	3	.006	2	.038	5	-1.868e-6	10	NC	1	NC	1
298			min	-.001	2	-.008	3	-.003	1	-1.191e-3	4	8167.362	2	NC	1
299		17	max	.001	3	.007	2	.041	5	-1.977e-6	10	NC	1	NC	1
300			min	-.001	2	-.008	3	-.003	1	-1.237e-3	4	6998.799	2	NC	1
301		18	max	.001	3	.008	2	.043	5	-2.085e-6	10	NC	3	NC	1
302			min	-.001	2	-.008	3	-.003	1	-1.283e-3	4	6090.098	2	NC	1
303		19	max	.002	3	.009	2	.046	5	-2.194e-6	10	NC	3	NC	1
304			min	-.002	2	-.008	3	-.003	1	-1.329e-3	4	5376.688	2	NC	1
305	M12	1	max	.001	1	.009	2	.003	1	3.538e-3	4	NC	1	NC	2
306			min	0	3	-.008	3	-.05	5	1.977e-6	10	NC	1	387.613	5
307		2	max	.001	1	.009	2	.003	1	3.538e-3	4	NC	1	NC	2
308			min	0	3	-.007	3	-.046	5	1.977e-6	10	NC	1	422.499	5
309		3	max	.001	1	.008	2	.002	1	3.538e-3	4	NC	1	NC	2
310			min	0	3	-.007	3	-.042	5	1.977e-6	10	NC	1	464.015	5
311		4	max	.001	1	.008	2	.002	1	3.538e-3	4	NC	1	NC	2
312			min	0	3	-.007	3	-.038	5	1.977e-6	10	NC	1	513.906	5
313		5	max	.001	1	.007	2	.002	1	3.538e-3	4	NC	1	NC	2
314			min	0	3	-.006	3	-.034	5	1.977e-6	10	NC	1	574.551	5
315		6	max	.001	1	.007	2	.002	1	3.538e-3	4	NC	1	NC	1
316			min	0	3	-.006	3	-.03	5	1.977e-6	10	NC	1	649.256	5
317		7	max	0	1	.006	2	.002	1	3.538e-3	4	NC	1	NC	1
318			min	0	3	-.005	3	-.026	5	1.977e-6	10	NC	1	742.733	5
319		8	max	0	1	.006	2	.001	1	3.538e-3	4	NC	1	NC	1
320			min	0	3	-.005	3	-.022	5	1.977e-6	10	NC	1	861.876	5
321		9	max	0	1	.005	2	.001	1	3.538e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.019	5	1.977e-6	10	NC	1	1017.115	5
323		10	max	0	1	.005	2	0	1	3.538e-3	4	NC	1	NC	1
324			min	0	3	-.004	3	-.016	5	1.977e-6	10	NC	1	1224.845	5
325		11	max	0	1	.004	2	0	1	3.538e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.013	5	1.977e-6	10	NC	1	1512.064	5
327		12	max	0	1	.004	2	0	1	3.538e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.01	5	1.977e-6	10	NC	1	1925.864	5
329		max	0	1	.003	2	0	1	3.538e-3	4	NC	1	NC	1
330		min	0	3	-.003	3	-.008	5	1.977e-6	10	NC	1	2554.57	5
331		max	0	1	.003	2	0	1	3.538e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.005	5	1.977e-6	10	NC	1	3580.362	5
333		max	0	1	.002	2	0	1	3.538e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.004	5	1.977e-6	10	NC	1	5431.212	5
335		max	0	1	.002	2	0	1	3.538e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.002	5	1.977e-6	10	NC	1	9323.672	5
337		max	0	1	.001	2	0	1	3.538e-3	4	NC	1	NC	1
338		min	0	3	0	3	0	5	1.977e-6	10	NC	1	NC	1
339		max	0	1	0	2	0	1	3.538e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	1.977e-6	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.538e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.977e-6	10	NC	1	NC	1
343	M1	max	.007	3	.023	3	.006	5	6.681e-3	2	NC	1	NC	1
344		min	-.008	2	-.02	2	-.001	9	-9.542e-3	3	NC	1	NC	1
345		max	.007	3	.013	3	.008	5	3.293e-3	2	NC	4	NC	1
346		min	-.008	2	-.011	2	-.002	1	-4.699e-3	3	4835.977	3	NC	1
347		max	.007	3	.004	3	.01	5	2.728e-4	5	NC	4	NC	1
348		min	-.008	2	-.003	2	-.003	1	-1.625e-4	1	2508.32	3	NC	1
349		max	.007	3	.004	2	.012	5	2.717e-4	5	NC	4	NC	1
350		min	-.008	2	-.004	3	-.004	1	-1.352e-4	1	1792.041	3	6888.377	5
351		max	.007	3	.011	2	.015	5	2.705e-4	5	NC	4	NC	1
352		min	-.008	2	-.01	3	-.004	1	-1.079e-4	1	1451.421	3	4900.972	5
353		max	.007	3	.016	2	.018	5	2.694e-4	5	NC	4	NC	1
354		min	-.008	2	-.015	3	-.004	1	-8.061e-5	1	1258.068	2	3748.425	5
355		max	.007	3	.02	2	.021	5	2.683e-4	5	NC	4	NC	1
356		min	-.008	2	-.019	3	-.003	1	-5.331e-5	1	1121.846	2	3006.543	5
357		max	.007	3	.023	2	.024	5	2.672e-4	5	NC	4	NC	1
358		min	-.008	2	-.022	3	-.003	1	-3.007e-5	9	1036.56	2	2495.164	5
359		max	.007	3	.025	2	.028	5	2.661e-4	5	NC	4	NC	1
360		min	-.008	2	-.023	3	-.002	1	-1.023e-5	9	986.194	2	2125.198	5
361		max	.007	3	.026	2	.031	5	2.705e-4	4	NC	4	NC	1
362		min	-.008	2	-.023	3	0	9	1.023e-6	10	963.121	2	1831.303	4
363		max	.007	3	.026	2	.034	4	2.759e-4	4	NC	4	NC	1
364		min	-.008	2	-.023	3	0	9	1.212e-6	10	964.607	2	1608.084	4
365		max	.007	3	.024	2	.038	4	2.812e-4	4	NC	4	NC	1
366		min	-.008	2	-.021	3	0	10	1.401e-6	10	991.796	2	1435.067	4
367		max	.007	3	.021	2	.041	4	2.866e-4	4	NC	4	NC	1
368		min	-.008	2	-.018	3	0	10	1.591e-6	10	1050.315	2	1298.73	4
369		max	.007	3	.017	2	.045	4	2.92e-4	4	NC	4	NC	1
370		min	-.008	2	-.014	3	0	10	1.78e-6	10	1153.119	2	1190.004	4
371		max	.007	3	.011	2	.048	4	2.974e-4	4	NC	4	NC	1
372		min	-.008	2	-.009	3	0	10	1.97e-6	10	1328.608	2	1102.631	4
373		max	.007	3	.004	2	.051	4	4.795e-4	4	NC	4	NC	1
374		min	-.008	2	-.003	3	0	10	2.117e-6	10	1646.138	2	1032.191	4
375		max	.007	3	.003	3	.053	4	4.869e-3	4	NC	4	NC	1
376		min	-.008	2	-.005	2	0	10	1.265e-6	10	2326.663	2	975.573	4
377		max	.007	3	.011	3	.056	4	4.622e-3	2	NC	4	NC	1
378		min	-.008	2	-.015	2	0	10	2.299e-3	3	4505.275	2	930.329	4
379		max	.007	3	.018	3	.058	4	9.313e-3	2	NC	1	NC	1
380		min	-.008	2	-.026	2	0	1	-4.698e-3	3	NC	1	895.784	4
381	M5	max	.022	3	.074	3	.005	5	1.329e-5	4	NC	1	NC	1
382		min	-.025	2	-.064	2	-.001	9	0	11	NC	1	NC	1
383		max	.022	3	.042	3	.007	5	1.354e-4	5	NC	4	NC	1
384		min	-.025	2	-.036	2	-.001	9	-1.975e-5	9	1529.063	3	NC	1



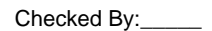
Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442			min	-.008	2	-.021	3	-.004	3	-1.327e-4	1	992.083	2	1959.056	5
443		13	max	.007	3	.021	2	.033	5	-1.495e-6	10	NC	4	NC	1
444			min	-.008	2	-.018	3	-.004	3	-1.565e-4	1	1050.625	2	1664.521	5
445		14	max	.007	3	.017	2	.037	5	-1.692e-6	10	NC	4	NC	1
446			min	-.008	2	-.014	3	-.003	3	-1.803e-4	1	1153.465	2	1444.514	5
447		15	max	.007	3	.011	2	.042	5	-1.89e-6	10	NC	4	NC	1
448			min	-.008	2	-.009	3	-.004	1	-2.041e-4	1	1329.01	2	1276.211	5
449		16	max	.007	3	.004	2	.046	5	1.481e-4	5	NC	4	NC	1
450			min	-.008	2	-.003	3	-.003	1	-2.225e-4	1	1646.631	2	1145.086	5
451		17	max	.007	3	.003	3	.05	5	4.843e-3	4	NC	4	NC	1
452			min	-.008	2	-.005	2	-.003	1	-1.104e-4	1	2327.31	2	1041.406	5
453		18	max	.007	3	.011	3	.054	5	2.378e-3	5	NC	4	NC	1
454			min	-.008	2	-.015	2	-.002	1	-4.622e-3	2	4506.486	2	954.16	4
455		19	max	.007	3	.018	3	.058	4	4.696e-3	3	NC	1	NC	1
456			min	-.008	2	-.026	2	0	9	-9.313e-3	2	NC	1	882.688	4
457	M13	1	max	.001	9	.023	3	.007	3	3.76e-3	3	NC	1	NC	1
458			min	-.005	5	-.02	2	-.008	2	-3.302e-3	2	NC	1	NC	1
459		2	max	.001	9	.075	3	.005	3	4.67e-3	3	NC	4	NC	1
460			min	-.005	5	-.056	2	-.006	2	-4.112e-3	2	1949.082	3	NC	1
461		3	max	.001	9	.119	3	.009	9	5.58e-3	3	NC	5	NC	2
462			min	-.005	5	-.087	2	-.005	2	-4.922e-3	2	1061.336	3	7912.701	1
463		4	max	.001	9	.148	3	.014	1	6.49e-3	3	NC	5	NC	2
464			min	-.005	5	-.109	2	-.005	10	-5.732e-3	2	812.757	3	5670.633	1
465		5	max	.001	9	.161	3	.015	1	7.4e-3	3	NC	5	NC	2
466			min	-.005	5	-.118	2	-.007	10	-6.542e-3	2	740.836	3	5273.049	1
467		6	max	.001	9	.156	3	.014	9	8.31e-3	3	NC	5	NC	2
468			min	-.005	5	-.116	2	-.009	2	-7.352e-3	2	767.99	3	6260.694	1
469		7	max	.001	9	.137	3	.014	3	9.22e-3	3	NC	5	NC	1
470			min	-.005	5	-.105	2	-.014	2	-8.162e-3	2	894.402	3	NC	1
471		8	max	.001	9	.111	3	.016	3	1.013e-2	3	NC	4	NC	1
472			min	-.005	5	-.088	2	-.019	2	-8.972e-3	2	1165.646	3	9149.272	2
473		9	max	.001	9	.085	3	.019	3	1.104e-2	3	NC	4	NC	1
474			min	-.005	5	-.071	2	-.023	2	-9.782e-3	2	1636.5	3	6552.551	2
475		10	max	.001	9	.074	3	.022	3	1.195e-2	3	NC	4	NC	4
476			min	-.005	5	-.064	2	-.025	2	-1.059e-2	2	2014.703	3	5824.413	2
477		11	max	.001	9	.086	3	.023	3	1.104e-2	3	NC	4	NC	1
478			min	-.005	5	-.071	2	-.023	2	-9.782e-3	2	1636.498	3	6280.576	3
479		12	max	.001	9	.111	3	.024	3	1.013e-2	3	NC	4	NC	1
480			min	-.005	5	-.088	2	-.019	2	-8.972e-3	2	1165.645	3	6107.53	3
481		13	max	.001	9	.137	3	.023	3	9.225e-3	3	NC	5	NC	1
482			min	-.005	5	-.105	2	-.014	2	-8.162e-3	2	894.401	3	6398.315	3
483		14	max	.001	9	.156	3	.021	3	8.317e-3	3	NC	5	NC	2
484			min	-.005	5	-.116	2	-.009	2	-7.352e-3	2	767.99	3	6255.933	1
485		15	max	.001	9	.161	3	.019	3	7.408e-3	3	NC	5	NC	2
486			min	-.005	5	-.118	2	-.007	10	-6.542e-3	2	740.836	3	5277.448	1
487		16	max	.001	9	.149	3	.016	3	6.5e-3	3	NC	5	NC	2
488			min	-.005	5	-.109	2	-.005	10	-5.732e-3	2	812.756	3	5683.328	1
489		17	max	.001	9	.119	3	.012	3	5.592e-3	3	NC	5	NC	2
490			min	-.005	5	-.087	2	-.005	2	-4.922e-3	2	1061.335	3	7944.266	1
491		18	max	.001	9	.076	3	.009	3	4.684e-3	3	NC	4	NC	1
492			min	-.006	5	-.056	2	-.006	2	-4.112e-3	2	1949.081	3	NC	1
493		19	max	.001	9	.023	3	.007	3	3.775e-3	3	NC	1	NC	1
494			min	-.006	5	-.02	2	-.008	2	-3.303e-3	2	NC	1	NC	1
495	M16	1	max	0	9	.018	3	.007	3	4.063e-3	2	NC	1	NC	1
496			min	-.058	4	-.026	2	-.008	2	-2.917e-3	3	NC	1	NC	1
497		2	max	0	9	.044	3	.01	3	5.065e-3	2	NC	4	NC	1
498			min	-.058	4	-.077	2	-.006	2	-3.597e-3	3	1996.432	2	NC	1





Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

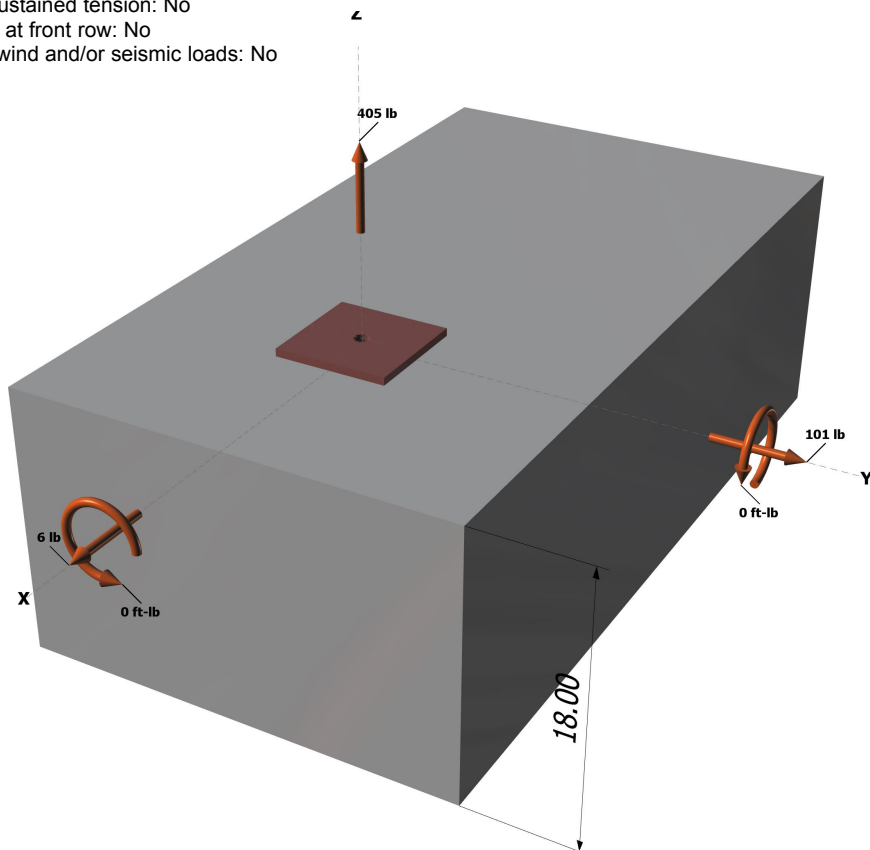
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



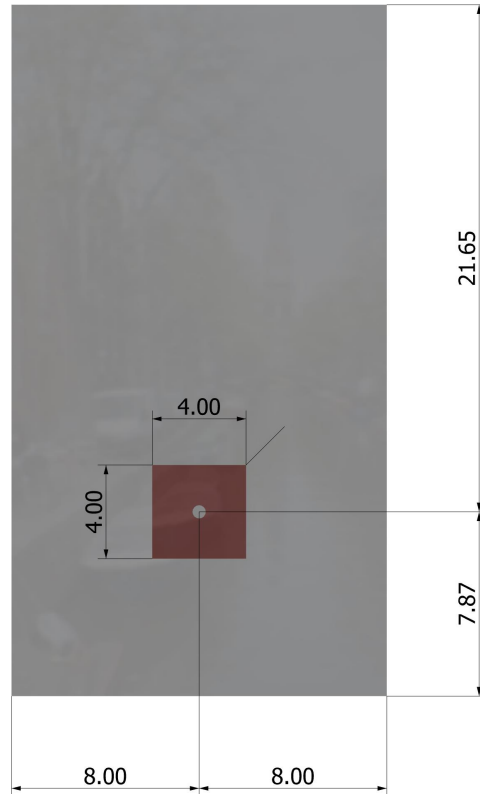
Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

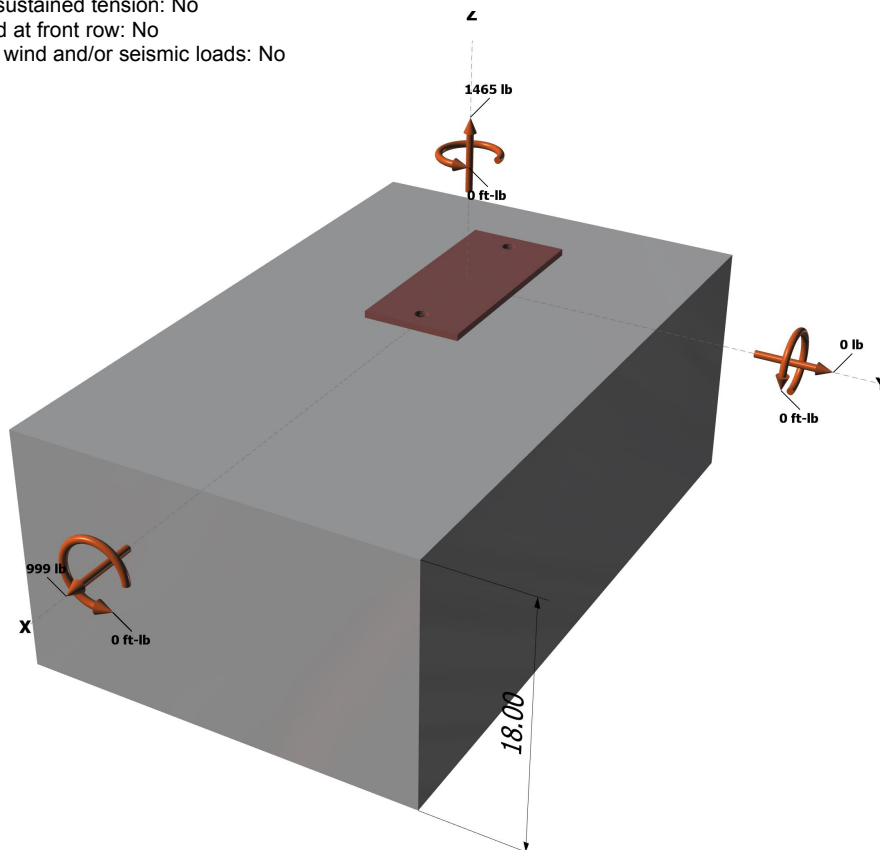
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

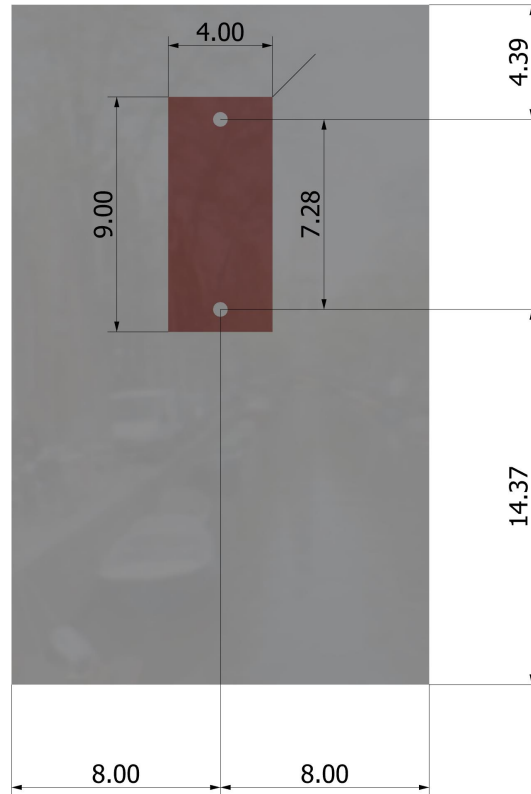
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5}$ (Eq. D-7)

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$ (Sec. D.4.1 & Eq. D-5)

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$ (Eq. D-16f)

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$ (Sec. D.4.1 & Eq. D-16b)

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.