



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 22.61 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	87 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.424 k-ft
M_z =	0.195 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	68%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

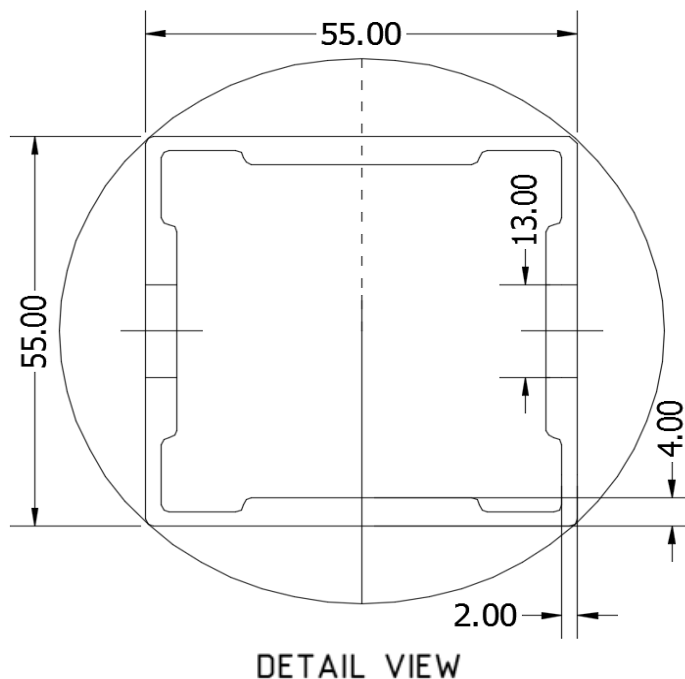
Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	104.56 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.00 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-3.380 k-ft
M_z =	0.000 k-ft
P_n =	-0.819 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	100%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

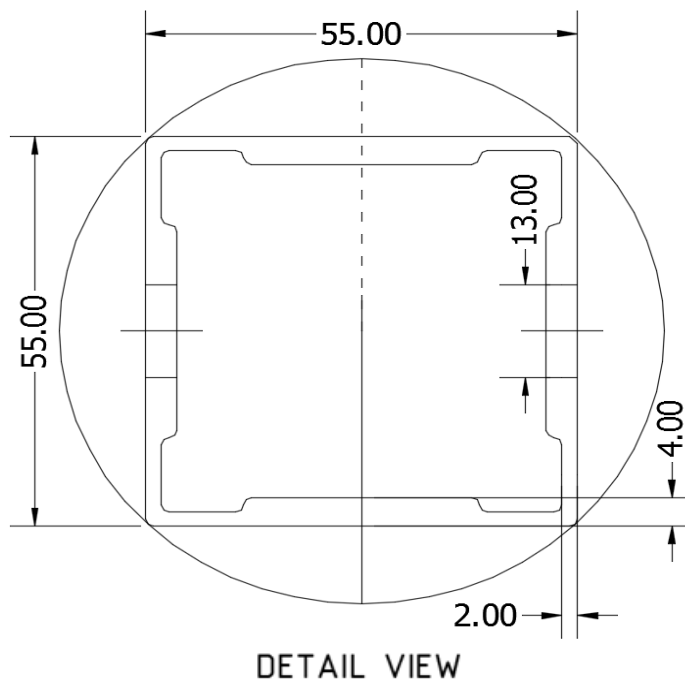
Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	3.131 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	11%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.013 k-ft
M_z =	0.000 k-ft
P_n =	1.987 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	34%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	61.10 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.63 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.013 k-ft
M_z =	0.000 k-ft
P_n =	3.276 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	13.386 k
Utilization =	<u>25%</u>



5. FOUNDATION DESIGN CALCULATIONS

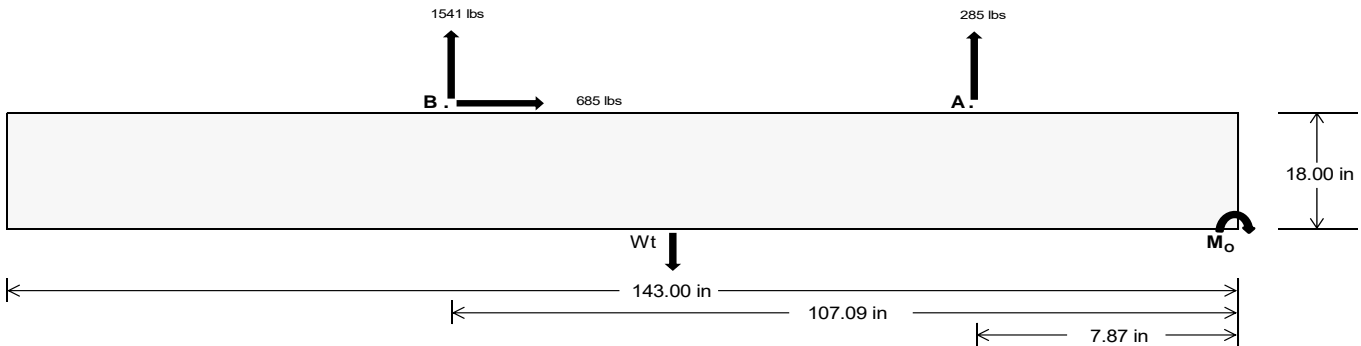
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>1197.69</u>	<u>6419.39</u> k
Compressive Load =		<u>4070.39</u>	<u>4894.76</u> k
Lateral Load =		<u>9.87</u>	<u>2848.59</u> k
Moment (Weak Axis) =		<u>0.02</u>	<u>0.00</u> k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 179599.9$ in-lbs
Resisting Force Required = 2511.89 lbs
S.F. = 1.67
Weight Required = 4186.48 lbs
Minimum Width = 35 in
Weight Provided = 7559.64 lbs

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Sliding

Force = 684.91 lbs
Friction = 0.4
Weight Required = 1712.29 lbs
Resisting Weight = 7559.64 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Cohesion

Sliding Force = 684.91 lbs
Cohesion = 130 psf
Area = 34.76 ft²
Resisting = 3779.82 lbs
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Shear key is not required.

Bearing Pressure

Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$ 35 in 36 in 37 in 38 in
7560 lbs 7776 lbs 7992 lbs 8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
F_A	1270 lbs	1270 lbs	1270 lbs	1270 lbs	1679 lbs	1679 lbs	1679 lbs	1679 lbs	2106 lbs	2106 lbs	2106 lbs	2106 lbs	-571 lbs	-571 lbs	-571 lbs	-571 lbs
F_B	1353 lbs	1353 lbs	1353 lbs	1353 lbs	2055 lbs	2055 lbs	2055 lbs	2055 lbs	2445 lbs	2445 lbs	2445 lbs	2445 lbs	-3082 lbs	-3082 lbs	-3082 lbs	-3082 lbs
F_V	121 lbs	121 lbs	121 lbs	121 lbs	1209 lbs	1209 lbs	1209 lbs	1209 lbs	988 lbs	988 lbs	988 lbs	988 lbs	-1370 lbs	-1370 lbs	-1370 lbs	-1370 lbs
P_{total}	10183 lbs	10399 lbs	10615 lbs	10831 lbs	11293 lbs	11509 lbs	11725 lbs	11941 lbs	12110 lbs	12326 lbs	12542 lbs	12758 lbs	883 lbs	1012 lbs	1142 lbs	1272 lbs
M	2902 lbs-ft	2902 lbs-ft	2902 lbs-ft	2902 lbs-ft	4621 lbs-ft	4621 lbs-ft	4621 lbs-ft	4621 lbs-ft	5399 lbs-ft	5399 lbs-ft	5399 lbs-ft	5399 lbs-ft	4058 lbs-ft	4058 lbs-ft	4058 lbs-ft	4058 lbs-ft
e	0.28 ft	0.28 ft	0.27 ft	0.27 ft	0.41 ft	0.40 ft	0.39 ft	0.39 ft	0.45 ft	0.44 ft	0.43 ft	0.42 ft	4.60 ft	4.01 ft	3.55 ft	3.19 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
f_{min}	250.9 psf	250.0 psf	249.1 psf	248.3 psf	258.0 psf	256.9 psf	255.8 psf	254.8 psf	270.2 psf	268.8 psf	267.4 psf	266.1 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	335.0 psf	331.7 psf	328.7 psf	325.7 psf	391.9 psf	387.0 psf	382.4 psf	378.1 psf	426.6 psf	420.8 psf	415.3 psf	410.1 psf	148.2 psf	115.4 psf	102.7 psf	96.8 psf

Maximum Bearing Pressure = 427 psf
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

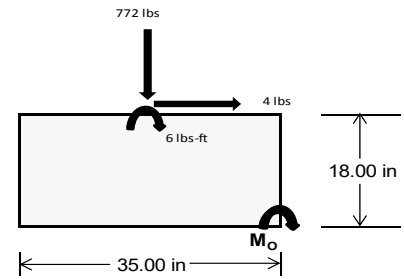
Overturning Check

$M_o = 1114.5 \text{ ft-lbs}$
 Resisting Force Required = 764.26 lbs
 S.F. = 1.67
 Weight Required = 1273.77 lbs
 Minimum Width = **35 in**
 Weight Provided = 7559.64 lbs

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	221 lbs	539 lbs	221 lbs	772 lbs	2154 lbs	772 lbs	65 lbs	158 lbs	65 lbs
F_v	1 lbs	0 lbs	1 lbs	4 lbs	0 lbs	4 lbs	0 lbs	0 lbs	0 lbs
P_{total}	9580 lbs	7560 lbs	9580 lbs	9681 lbs	7560 lbs	9681 lbs	2801 lbs	7560 lbs	2801 lbs
M	3 lbs-ft	0 lbs-ft	3 lbs-ft	12 lbs-ft	0 lbs-ft	12 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
f_{min}	275.4 psf	217.5 psf	275.4 psf	277.8 psf	217.5 psf	277.8 psf	80.6 psf	217.5 psf	80.6 psf
f_{max}	275.8 psf	217.5 psf	275.8 psf	279.2 psf	217.5 psf	279.2 psf	80.6 psf	217.5 psf	80.6 psf



Maximum Bearing Pressure = 279 psf
 Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 143in long x 34in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.963 k
Allowable Uplift =	1.214 k
Utilization =	<u>79%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.493 k
Allowable Uplift =	4.357 k
Utilization =	<u>57%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	3.131 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>42%</u>

Rear Strut

Maximum Axial Load =	4.420 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>60%</u>

Diagonal Strut

Maximum Axial Load =	2.183 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>29%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	51.89 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	1.038 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 87 \text{ in}$$

$$J = 0.432$$

$$240.683$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 87$$

$$J = 0.432$$

$$153.06$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned}L_b &= 98.03 \text{ in} \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4 \text{ ksi}\end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned}L_b &= 98.03 \\ J &= 0.942 \\ &= 152.985 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.4\end{aligned}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 61.10 \text{ in} \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 61.1 \\ J &= 0.942 \\ &= 95.3524 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= 1.17 \phi_y Fcy \\ \phi F_L &= 38.9 \text{ ksi} \end{aligned}$$

3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 28.2 \text{ ksi} \\ I_x &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ y &= 27.5 \text{ mm} \\ S_x &= 0.621 \text{ in}^3 \\ M_{max} St &= 1.460 \text{ k-ft} \end{aligned}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned} h/t &= 24.5 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 27.5 \\ Cc &= 27.5 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3 \phi_y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 28.2 \text{ ksi} \\ I_y &= 279836 \text{ mm}^4 \\ &= 0.672 \text{ in}^4 \\ x &= 27.5 \text{ mm} \\ S_y &= 0.621 \text{ in}^3 \\ M_{max} Wk &= 1.460 \text{ k-ft} \end{aligned}$$

Compression

3.4.7

$$\begin{aligned} \lambda &= 1.41345 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77788 \\ \phi F_L &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi F_L &= 13.6277 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 13.63 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 14.03 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	Y	-63.565	-63.565	0	0
2	M14	Y	-63.565	-63.565	0	0
3	M15	Y	-63.565	-63.565	0	0
4	M16	Y	-63.565	-63.565	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	-77.887	-77.887	0	0
2	M14	y	-77.887	-77.887	0	0
3	M15	y	-122.393	-122.393	0	0
4	M16	y	-122.393	-122.393	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft,%]	End Location[ft,%]
1	M13	y	178.027	178.027	0	0
2	M14	y	136.487	136.487	0	0
3	M15	y	74.178	74.178	0	0
4	M16	y	74.178	74.178	0	0

Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 20° 120mph 30psf 7.25ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	46.984	1	234.872	1	1.701	3	.014	2	-.003	15	.814	3
28			min	1.72	15	-359.617	3	-24.975	1	0	15	-.089	1	-.438	1
29		15	max	46.984	1	97.404	2	6.797	1	.014	2	-.003	12	1.013	3
30			min	1.72	15	-134.9	3	-.547	10	0	15	-.096	1	-.572	1
31		16	max	46.984	1	89.816	3	38.57	1	.014	2	0	12	1.032	3
32			min	1.72	15	-40.105	1	1.412	15	0	15	-.078	1	-.595	1
33		17	max	46.984	1	314.533	3	70.343	1	.014	2	.004	3	.869	3
34			min	1.72	15	-177.594	1	2.544	15	0	15	-.034	1	-.508	1
35		18	max	46.984	1	539.25	3	102.115	1	.014	2	.036	1	.525	3
36			min	1.72	15	-315.083	1	3.675	15	0	15	0	10	-.309	1
37		19	max	46.984	1	763.966	3	133.888	1	.014	2	.131	1	0	1
38			min	1.72	15	-452.571	1	4.807	15	0	15	.005	15	0	3
39	M14	1	max	31.336	1	532.424	1	-5.027	15	.015	3	.16	1	0	1
40			min	1.143	15	-631.848	3	-140.004	1	-.016	2	.006	15	0	3
41		2	max	31.336	1	394.936	1	-3.895	15	.015	3	.06	1	.44	3
42			min	1.143	15	-460.671	3	-108.231	1	-.016	2	.002	15	-.374	1
43		3	max	31.336	1	257.974	2	-2.763	15	.015	3	.005	3	.742	3
44			min	1.143	15	-289.495	3	-76.459	1	-.016	2	-.014	1	-.636	1
45		4	max	31.336	1	123.221	2	-1.632	15	.015	3	0	3	.906	3
46			min	1.143	15	-118.319	3	-44.686	1	-.016	2	-.063	1	-.788	1
47		5	max	31.336	1	52.857	3	.018	10	.015	3	-.002	12	.933	3
48			min	1.143	15	-17.53	1	-12.913	1	-.016	2	-.086	1	-.831	2
49		6	max	31.336	1	224.033	3	18.859	1	.015	3	-.003	15	.821	3
50			min	1.143	15	-155.019	1	-2.124	3	-.016	2	-.084	1	-.768	2
51		7	max	31.336	1	395.209	3	50.632	1	.015	3	-.002	15	.572	3
52			min	1.143	15	-292.507	1	-.398	3	-.016	2	-.056	1	-.596	2
53		8	max	31.336	1	566.385	3	82.405	1	.015	3	.003	2	.185	3
54			min	1.143	15	-429.996	1	1.01	12	-.016	2	-.007	3	-.315	2
55		9	max	31.336	1	737.562	3	114.177	1	.015	3	.077	1	.113	1
56			min	1.143	15	-567.484	1	2.16	12	-.016	2	-.005	3	-.341	3
57		10	max	31.336	1	908.738	3	145.95	1	.015	3	.182	1	.625	1
58			min	1.143	15	-704.973	1	3.31	12	-.016	2	-.002	3	-1.004	3
59		11	max	31.336	1	567.484	1	-2.16	12	.016	2	.077	1	.113	1
60			min	1.143	15	-737.562	3	-114.177	1	-.015	3	-.005	3	-.341	3
61		12	max	31.336	1	429.996	1	-1.01	12	.016	2	.003	2	.185	3
62			min	1.143	15	-566.385	3	-82.405	1	-.015	3	-.007	3	-.315	2
63		13	max	31.336	1	292.507	1	.398	3	.016	2	-.002	15	.572	3
64			min	1.143	15	-395.209	3	-50.632	1	-.015	3	-.056	1	-.596	2
65		14	max	31.336	1	155.019	1	2.124	3	.016	2	-.003	15	.821	3
66			min	1.143	15	-224.033	3	-18.859	1	-.015	3	-.084	1	-.768	2
67		15	max	31.336	1	17.53	1	12.913	1	.016	2	-.002	12	.933	3
68			min	1.143	15	-52.857	3	-.018	10	-.015	3	-.086	1	-.831	2
69		16	max	31.336	1	118.319	3	44.686	1	.016	2	0	3	.906	3
70			min	1.143	15	-123.221	2	1.632	15	-.015	3	-.063	1	-.788	1
71		17	max	31.336	1	289.495	3	76.459	1	.016	2	.005	3	.742	3
72			min	1.143	15	-257.974	2	2.763	15	-.015	3	-.014	1	-.636	1
73		18	max	31.336	1	460.671	3	108.231	1	.016	2	.06	1	.44	3
74			min	1.143	15	-394.936	1	3.895	15	-.015	3	.002	15	-.374	1
75		19	max	31.336	1	631.848	3	140.004	1	.016	2	.16	1	0	1
76			min	1.143	15	-532.424	1	5.027	15	-.015	3	.006	15	0	3
77	M15	1	max	-1.205	15	721.291	2	-5.025	15	.017	2	.16	1	0	2
78			min	-32.816	1	-361.977	3	-140.02	1	-.012	3	.006	15	0	3
79		2	max	-1.205	15	529.174	2	-3.893	15	.017	2	.06	1	.255	3
80			min	-32.816	1	-271.11	3	-108.248	1	-.012	3	.002	15	-.504	2
81		3	max	-1.205	15	337.057	2	-2.761	15	.017	2	.005	3	.437	3
82			min	-32.816	1	-180.243	3	-76.475	1	-.012	3	-.014	1	-.853	2
83		4	max	-1.205	15	144.94	2	-1.63	15	.017	2	0	3	.545	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-32.816	1	-89.377	3	-44.702	1	-.012	3	-.063	1	-1.047	2
85		5	max	-1.205	15	1.625	12	-.065	10	.017	2	-.002	12	.581	3
86			min	-32.816	1	-47.177	2	-12.93	1	-.012	3	-.086	1	-1.086	2
87		6	max	-1.205	15	92.357	3	18.843	1	.017	2	-.003	15	.543	3
88			min	-32.816	1	-239.294	2	-1.86	3	-.012	3	-.084	1	-.971	2
89		7	max	-1.205	15	183.224	3	50.616	1	.017	2	-.002	15	.432	3
90			min	-32.816	1	-431.411	2	-.134	3	-.012	3	-.056	1	-.701	2
91		8	max	-1.205	15	274.09	3	82.388	1	.017	2	.002	2	.248	3
92			min	-32.816	1	-623.528	2	1.175	12	-.012	3	-.006	3	-.276	2
93		9	max	-1.205	15	364.957	3	114.161	1	.017	2	.077	1	.304	2
94			min	-32.816	1	-815.645	2	2.326	12	-.012	3	-.004	3	-.01	12
95		10	max	-1.205	15	455.824	3	145.934	1	.012	3	.181	1	1.038	2
96			min	-32.816	1	-1007.762	2	3.476	12	-.017	2	0	3	-.34	3
97		11	max	-1.205	15	815.645	2	-2.326	12	.012	3	.077	1	.304	2
98			min	-32.816	1	-364.957	3	-114.161	1	-.017	2	-.004	3	-.01	12
99		12	max	-1.205	15	623.528	2	-1.175	12	.012	3	.002	2	.248	3
100			min	-32.816	1	-274.09	3	-82.388	1	-.017	2	-.006	3	-.276	2
101		13	max	-1.205	15	431.411	2	.134	3	.012	3	-.002	15	.432	3
102			min	-32.816	1	-183.224	3	-50.616	1	-.017	2	-.056	1	-.701	2
103		14	max	-1.205	15	239.294	2	1.86	3	.012	3	-.003	15	.543	3
104			min	-32.816	1	-92.357	3	-18.843	1	-.017	2	-.084	1	-.971	2
105		15	max	-1.205	15	47.177	2	12.93	1	.012	3	-.002	12	.581	3
106			min	-32.816	1	-1.625	12	.065	10	-.017	2	-.086	1	-1.086	2
107		16	max	-1.205	15	89.377	3	44.702	1	.012	3	0	3	.545	3
108			min	-32.816	1	-144.94	2	1.63	15	-.017	2	-.063	1	-1.047	2
109		17	max	-1.205	15	180.243	3	76.475	1	.012	3	.005	3	.437	3
110			min	-32.816	1	-337.057	2	2.761	15	-.017	2	-.014	1	-.853	2
111		18	max	-1.205	15	271.11	3	108.248	1	.012	3	.06	1	.255	3
112			min	-32.816	1	-529.174	2	3.893	15	-.017	2	.002	15	-.504	2
113		19	max	-1.205	15	361.977	3	140.02	1	.012	3	.16	1	0	2
114			min	-32.816	1	-721.291	2	5.025	15	-.017	2	.006	15	0	3
115	M16	1	max	-1.918	15	640.025	2	-4.82	15	.008	1	.133	1	0	2
116			min	-52.503	1	-294.168	3	-134.476	1	-.012	3	.005	15	0	3
117		2	max	-1.918	15	447.908	2	-3.688	15	.008	1	.037	1	.2	3
118			min	-52.503	1	-203.302	3	-102.703	1	-.012	3	.001	10	-.438	2
119		3	max	-1.918	15	255.792	2	-2.556	15	.008	1	.002	3	.328	3
120			min	-52.503	1	-112.435	3	-70.931	1	-.012	3	-.032	1	-.722	2
121		4	max	-1.918	15	63.675	2	-1.425	15	.008	1	-.001	12	.382	3
122			min	-52.503	1	-21.568	3	-39.158	1	-.012	3	-.077	1	-.85	2
123		5	max	-1.918	15	69.299	3	.229	10	.008	1	-.003	12	.362	3
124			min	-52.503	1	-128.442	2	-7.385	1	-.012	3	-.096	1	-.824	2
125		6	max	-1.918	15	160.165	3	24.387	1	.008	1	-.003	15	.27	3
126			min	-52.503	1	-320.559	2	-.868	3	-.012	3	-.089	1	-.643	2
127		7	max	-1.918	15	251.032	3	56.16	1	.008	1	-.002	15	.104	3
128			min	-52.503	1	-512.676	2	.658	12	-.012	3	-.056	1	-.308	2
129		8	max	-1.918	15	341.899	3	87.933	1	.008	1	.004	2	.183	2
130			min	-52.503	1	-704.793	2	1.809	12	-.012	3	-.005	3	-.135	3
131		9	max	-1.918	15	432.766	3	119.706	1	.008	1	.085	1	.828	2
132			min	-52.503	1	-896.91	2	2.959	12	-.012	3	-.002	3	-.447	3
133		10	max	-1.918	15	523.633	3	151.478	1	.008	1	.195	1	1.628	2
134			min	-52.503	1	-1089.027	2	4.109	12	-.012	3	.002	12	-.832	3
135		11	max	-1.918	15	896.91	2	-2.959	12	.012	3	.085	1	.828	2
136			min	-52.503	1	-432.766	3	-119.706	1	-.008	1	-.002	3	-.447	3
137		12	max	-1.918	15	704.793	2	-1.809	12	.012	3	.004	2	.183	2
138			min	-52.503	1	-341.899	3	-87.933	1	-.008	1	-.005	3	-.135	3
139		13	max	-1.918	15	512.676	2	-.658	12	.012	3	-.002	15	.104	3
140			min	-52.503	1	-251.032	3	-56.16	1	-.008	1	-.056	1	-.308	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141	14	max	-1.918	15	320.559	2	.868	3	.012	3	-.003	15	.27	3
142		min	-52.503	1	-160.165	3	-24.387	1	-.008	1	-.089	1	-.643	2
143	15	max	-1.918	15	128.442	2	7.385	1	.012	3	-.003	12	.362	3
144		min	-52.503	1	-69.299	3	-.229	10	-.008	1	-.096	1	-.824	2
145	16	max	-1.918	15	21.568	3	39.158	1	.012	3	-.001	12	.382	3
146		min	-52.503	1	-63.675	2	1.425	15	-.008	1	-.077	1	-.85	2
147	17	max	-1.918	15	112.435	3	70.931	1	.012	3	.002	3	.328	3
148		min	-52.503	1	-255.792	2	2.556	15	-.008	1	-.032	1	-.722	2
149	18	max	-1.918	15	203.302	3	102.703	1	.012	3	.037	1	.2	3
150		min	-52.503	1	-447.908	2	3.688	15	-.008	1	.001	10	-.438	2
151	19	max	-1.918	15	294.168	3	134.476	1	.012	3	.133	1	0	2
152		min	-52.503	1	-640.025	2	4.82	15	-.008	1	.005	15	0	3
153	M2	1	max	1089.048	2	2.158	4	.551	1	0	3	0	3	1
154		min	-1427.791	3	.507	15	.02	15	0	1	0	1	0	1
155	2	max	1089.464	2	2.149	4	.551	1	0	3	0	1	0	15
156		min	-1427.479	3	.505	15	.02	15	0	1	0	15	0	4
157	3	max	1089.88	2	2.141	4	.551	1	0	3	0	1	0	15
158		min	-1427.167	3	.503	15	.02	15	0	1	0	15	-.001	4
159	4	max	1090.296	2	2.132	4	.551	1	0	3	0	1	0	15
160		min	-1426.855	3	.501	15	.02	15	0	1	0	15	-.002	4
161	5	max	1090.712	2	2.123	4	.551	1	0	3	0	1	0	15
162		min	-1426.543	3	.499	15	.02	15	0	1	0	15	-.002	4
163	6	max	1091.127	2	2.114	4	.551	1	0	3	0	1	0	15
164		min	-1426.231	3	.497	15	.02	15	0	1	0	15	-.003	4
165	7	max	1091.543	2	2.106	4	.551	1	0	3	0	1	0	15
166		min	-1425.92	3	.495	15	.02	15	0	1	0	15	-.004	4
167	8	max	1091.959	2	2.097	4	.551	1	0	3	.001	1	0	15
168		min	-1425.608	3	.493	15	.02	15	0	1	0	15	-.004	4
169	9	max	1092.375	2	2.088	4	.551	1	0	3	.001	1	-.001	15
170		min	-1425.296	3	.491	15	.02	15	0	1	0	15	-.005	4
171	10	max	1092.791	2	2.08	4	.551	1	0	3	.001	1	-.001	15
172		min	-1424.984	3	.489	15	.02	15	0	1	0	15	-.005	4
173	11	max	1093.207	2	2.071	4	.551	1	0	3	.002	1	-.001	15
174		min	-1424.672	3	.487	15	.02	15	0	1	0	15	-.006	4
175	12	max	1093.623	2	2.062	4	.551	1	0	3	.002	1	-.002	15
176		min	-1424.36	3	.485	15	.02	15	0	1	0	15	-.007	4
177	13	max	1094.039	2	2.053	4	.551	1	0	3	.002	1	-.002	15
178		min	-1424.048	3	.483	15	.02	15	0	1	0	15	-.007	4
179	14	max	1094.455	2	2.045	4	.551	1	0	3	.002	1	-.002	15
180		min	-1423.736	3	.481	15	.02	15	0	1	0	15	-.008	4
181	15	max	1094.87	2	2.036	4	.551	1	0	3	.002	1	-.002	15
182		min	-1423.424	3	.479	15	.02	15	0	1	0	15	-.008	4
183	16	max	1095.286	2	2.027	4	.551	1	0	3	.002	1	-.002	15
184		min	-1423.112	3	.476	15	.02	15	0	1	0	15	-.009	4
185	17	max	1095.702	2	2.019	4	.551	1	0	3	.002	1	-.002	15
186		min	-1422.801	3	.474	15	.02	15	0	1	0	15	-.009	4
187	18	max	1096.118	2	2.01	4	.551	1	0	3	.003	1	-.002	15
188		min	-1422.489	3	.472	15	.02	15	0	1	0	15	-.01	4
189	19	max	1096.534	2	2.001	4	.551	1	0	3	.003	1	-.002	15
190		min	-1422.177	3	.47	15	.02	15	0	1	0	15	-.01	4
191	M3	1	max	600.629	2	9.101	4	.137	1	0	3	0	1	4
192		min	-738.36	3	2.139	15	.005	15	0	1	0	15	.002	15
193	2	max	600.458	2	8.227	4	.137	1	0	3	0	1	.006	2
194		min	-738.488	3	1.934	15	.005	15	0	1	0	15	.001	12
195	3	max	600.288	2	7.353	4	.137	1	0	3	0	1	.003	2
196		min	-738.616	3	1.728	15	.005	15	0	1	0	15	0	3
197	4	max	600.118	2	6.478	4	.137	1	0	3	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-738.744	3	1.523	15	.005	15	0	1	0	15	-.002	3
199	5	max	599.947	2	5.604	4	.137	1	0	3	0	1	0	15
200		min	-738.871	3	1.317	15	.005	15	0	1	0	15	-.004	3
201	6	max	599.777	2	4.729	4	.137	1	0	3	0	1	-.001	15
202		min	-738.999	3	1.112	15	.005	15	0	1	0	15	-.006	4
203	7	max	599.606	2	3.855	4	.137	1	0	3	0	1	-.002	15
204		min	-739.127	3	.906	15	.005	15	0	1	0	15	-.008	4
205	8	max	599.436	2	2.98	4	.137	1	0	3	0	1	-.002	15
206		min	-739.255	3	.701	15	.005	15	0	1	0	15	-.01	4
207	9	max	599.266	2	2.106	4	.137	1	0	3	0	1	-.003	15
208		min	-739.382	3	.495	15	.005	15	0	1	0	15	-.011	4
209	10	max	599.095	2	1.231	4	.137	1	0	3	0	1	-.003	15
210		min	-739.51	3	.289	15	.005	15	0	1	0	15	-.012	4
211	11	max	598.925	2	.455	2	.137	1	0	3	0	1	-.003	15
212		min	-739.638	3	-.035	3	.005	15	0	1	0	15	-.012	4
213	12	max	598.755	2	-.122	15	.137	1	0	3	0	1	-.003	15
214		min	-739.766	3	-.546	3	.005	15	0	1	0	15	-.012	4
215	13	max	598.584	2	-.327	15	.137	1	0	3	0	1	-.003	15
216		min	-739.893	3	-1.392	4	.005	15	0	1	0	15	-.011	4
217	14	max	598.414	2	-.533	15	.137	1	0	3	0	1	-.002	15
218		min	-740.021	3	-2.266	4	.005	15	0	1	0	15	-.011	4
219	15	max	598.244	2	-.738	15	.137	1	0	3	0	1	-.002	15
220		min	-740.149	3	-3.141	4	.005	15	0	1	0	15	-.009	4
221	16	max	598.073	2	-.944	15	.137	1	0	3	.001	1	-.002	15
222		min	-740.277	3	-4.015	4	.005	15	0	1	0	15	-.008	4
223	17	max	597.903	2	-1.149	15	.137	1	0	3	.001	1	-.001	15
224		min	-740.404	3	-4.89	4	.005	15	0	1	0	15	-.005	4
225	18	max	597.733	2	-1.355	15	.137	1	0	3	.001	1	0	15
226		min	-740.532	3	-5.764	4	.005	15	0	1	0	15	-.003	4
227	19	max	597.562	2	-1.561	15	.137	1	0	3	.001	1	0	1
228		min	-740.66	3	-6.638	4	.005	15	0	1	0	15	0	1
229	M4	1	max	1124.511	1	0	1	-.285	15	0	1	0	1	0
230		min	-280.18	3	0	1	-7.851	1	0	1	0	15	0	1
231	2	max	1124.681	1	0	1	-.285	15	0	1	0	12	0	1
232		min	-280.052	3	0	1	-7.851	1	0	1	0	1	0	1
233	3	max	1124.852	1	0	1	-.285	15	0	1	0	15	0	1
234		min	-279.924	3	0	1	-7.851	1	0	1	-.001	1	0	1
235	4	max	1125.022	1	0	1	-.285	15	0	1	0	15	0	1
236		min	-279.797	3	0	1	-7.851	1	0	1	-.002	1	0	1
237	5	max	1125.192	1	0	1	-.285	15	0	1	0	15	0	1
238		min	-279.669	3	0	1	-7.851	1	0	1	-.003	1	0	1
239	6	max	1125.363	1	0	1	-.285	15	0	1	0	15	0	1
240		min	-279.541	3	0	1	-7.851	1	0	1	-.004	1	0	1
241	7	max	1125.533	1	0	1	-.285	15	0	1	0	15	0	1
242		min	-279.413	3	0	1	-7.851	1	0	1	-.005	1	0	1
243	8	max	1125.703	1	0	1	-.285	15	0	1	0	15	0	1
244		min	-279.286	3	0	1	-7.851	1	0	1	-.006	1	0	1
245	9	max	1125.874	1	0	1	-.285	15	0	1	0	15	0	1
246		min	-279.158	3	0	1	-7.851	1	0	1	-.006	1	0	1
247	10	max	1126.044	1	0	1	-.285	15	0	1	0	15	0	1
248		min	-279.03	3	0	1	-7.851	1	0	1	-.007	1	0	1
249	11	max	1126.214	1	0	1	-.285	15	0	1	0	15	0	1
250		min	-278.902	3	0	1	-7.851	1	0	1	-.008	1	0	1
251	12	max	1126.385	1	0	1	-.285	15	0	1	0	15	0	1
252		min	-278.775	3	0	1	-7.851	1	0	1	-.009	1	0	1
253	13	max	1126.555	1	0	1	-.285	15	0	1	0	15	0	1
254		min	-278.647	3	0	1	-7.851	1	0	1	-.01	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1126.725	1	0	1	-.285	15	0	1	0	15	0	1
256		min	-278.519	3	0	1	-7.851	1	0	1	-.011	1	0	1
257	15	max	1126.896	1	0	1	-.285	15	0	1	0	15	0	1
258		min	-278.391	3	0	1	-7.851	1	0	1	-.012	1	0	1
259	16	max	1127.066	1	0	1	-.285	15	0	1	0	15	0	1
260		min	-278.264	3	0	1	-7.851	1	0	1	-.013	1	0	1
261	17	max	1127.236	1	0	1	-.285	15	0	1	0	15	0	1
262		min	-278.136	3	0	1	-7.851	1	0	1	-.014	1	0	1
263	18	max	1127.407	1	0	1	-.285	15	0	1	0	15	0	1
264		min	-278.008	3	0	1	-7.851	1	0	1	-.015	1	0	1
265	19	max	1127.577	1	0	1	-.285	15	0	1	0	15	0	1
266		min	-277.88	3	0	1	-7.851	1	0	1	-.016	1	0	1
267	M6	1	max	3268.948	2	2.648	2	0	1	0	0	1	0	1
268		min	-4419.961	3	.03	3	0	1	0	1	0	1	0	1
269	2	max	3269.364	2	2.642	2	0	1	0	1	0	1	0	3
270		min	-4419.649	3	.025	3	0	1	0	1	0	1	0	2
271	3	max	3269.78	2	2.635	2	0	1	0	1	0	1	0	3
272		min	-4419.337	3	.02	3	0	1	0	1	0	1	-.001	2
273	4	max	3270.195	2	2.628	2	0	1	0	1	0	1	0	3
274		min	-4419.025	3	.015	3	0	1	0	1	0	1	-.002	2
275	5	max	3270.611	2	2.621	2	0	1	0	1	0	1	0	3
276		min	-4418.713	3	.01	3	0	1	0	1	0	1	-.003	2
277	6	max	3271.027	2	2.614	2	0	1	0	1	0	1	0	3
278		min	-4418.401	3	.004	3	0	1	0	1	0	1	-.004	2
279	7	max	3271.443	2	2.608	2	0	1	0	1	0	1	0	3
280		min	-4418.089	3	0	3	0	1	0	1	0	1	-.004	2
281	8	max	3271.859	2	2.601	2	0	1	0	1	0	1	0	3
282		min	-4417.777	3	-.006	3	0	1	0	1	0	1	-.005	2
283	9	max	3272.275	2	2.594	2	0	1	0	1	0	1	0	3
284		min	-4417.465	3	-.011	3	0	1	0	1	0	1	-.006	2
285	10	max	3272.691	2	2.587	2	0	1	0	1	0	1	0	3
286		min	-4417.153	3	-.016	3	0	1	0	1	0	1	-.007	2
287	11	max	3273.107	2	2.58	2	0	1	0	1	0	1	0	3
288		min	-4416.841	3	-.021	3	0	1	0	1	0	1	-.007	2
289	12	max	3273.522	2	2.574	2	0	1	0	1	0	1	0	3
290		min	-4416.53	3	-.026	3	0	1	0	1	0	1	-.008	2
291	13	max	3273.938	2	2.567	2	0	1	0	1	0	1	0	3
292		min	-4416.218	3	-.031	3	0	1	0	1	0	1	-.009	2
293	14	max	3274.354	2	2.56	2	0	1	0	1	0	1	0	3
294		min	-4415.906	3	-.036	3	0	1	0	1	0	1	-.009	2
295	15	max	3274.77	2	2.553	2	0	1	0	1	0	1	0	3
296		min	-4415.594	3	-.041	3	0	1	0	1	0	1	-.01	2
297	16	max	3275.186	2	2.546	2	0	1	0	1	0	1	0	3
298		min	-4415.282	3	-.047	3	0	1	0	1	0	1	-.011	2
299	17	max	3275.602	2	2.54	2	0	1	0	1	0	1	0	3
300		min	-4414.97	3	-.052	3	0	1	0	1	0	1	-.012	2
301	18	max	3276.018	2	2.533	2	0	1	0	1	0	1	0	3
302		min	-4414.658	3	-.057	3	0	1	0	1	0	1	-.012	2
303	19	max	3276.434	2	2.526	2	0	1	0	1	0	1	0	3
304		min	-4414.346	3	-.062	3	0	1	0	1	0	1	-.013	2
305	M7	1	max	1986.655	2	9.132	4	0	1	0	0	1	.013	2
306		min	-2180.277	3	2.144	15	0	1	0	1	0	1	0	3
307	2	max	1986.485	2	8.257	4	0	1	0	1	0	1	.01	2
308		min	-2180.405	3	1.938	15	0	1	0	1	0	1	-.002	3
309	3	max	1986.315	2	7.383	4	0	1	0	1	0	1	.006	2
310		min	-2180.533	3	1.732	15	0	1	0	1	0	1	-.004	3
311	4	max	1986.144	2	6.508	4	0	1	0	1	0	1	.004	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2180.66	3	1.527	15	0	1	0	1	0	1	-.006	3
313	5	max	1985.974	2	5.634	4	0	1	0	1	0	1	.001	2
314		min	-2180.788	3	1.321	15	0	1	0	1	0	1	-.007	3
315	6	max	1985.804	2	4.759	4	0	1	0	1	0	1	-.001	2
316		min	-2180.916	3	1.116	15	0	1	0	1	0	1	-.008	3
317	7	max	1985.633	2	3.885	4	0	1	0	1	0	1	-.002	15
318		min	-2181.044	3	.91	15	0	1	0	1	0	1	-.009	3
319	8	max	1985.463	2	3.011	4	0	1	0	1	0	1	-.002	15
320		min	-2181.171	3	.705	15	0	1	0	1	0	1	-.009	4
321	9	max	1985.293	2	2.214	2	0	1	0	1	0	1	-.003	15
322		min	-2181.299	3	.365	12	0	1	0	1	0	1	-.011	4
323	10	max	1985.122	2	1.532	2	0	1	0	1	0	1	-.003	15
324		min	-2181.427	3	-.009	3	0	1	0	1	0	1	-.011	4
325	11	max	1984.952	2	.851	2	0	1	0	1	0	1	-.003	15
326		min	-2181.555	3	-.52	3	0	1	0	1	0	1	-.012	4
327	12	max	1984.782	2	.169	2	0	1	0	1	0	1	-.003	15
328		min	-2181.683	3	-1.032	3	0	1	0	1	0	1	-.012	4
329	13	max	1984.611	2	-.323	15	0	1	0	1	0	1	-.003	15
330		min	-2181.81	3	-1.543	3	0	1	0	1	0	1	-.011	4
331	14	max	1984.441	2	-.529	15	0	1	0	1	0	1	-.002	15
332		min	-2181.938	3	-2.236	4	0	1	0	1	0	1	-.01	4
333	15	max	1984.271	2	-.734	15	0	1	0	1	0	1	-.002	15
334		min	-2182.066	3	-3.111	4	0	1	0	1	0	1	-.009	4
335	16	max	1984.1	2	-.94	15	0	1	0	1	0	1	-.002	15
336		min	-2182.194	3	-3.985	4	0	1	0	1	0	1	-.008	4
337	17	max	1983.93	2	-1.145	15	0	1	0	1	0	1	-.001	15
338		min	-2182.321	3	-4.859	4	0	1	0	1	0	1	-.005	4
339	18	max	1983.76	2	-1.351	15	0	1	0	1	0	1	0	15
340		min	-2182.449	3	-5.734	4	0	1	0	1	0	1	-.003	4
341	19	max	1983.589	2	-1.556	15	0	1	0	1	0	1	0	1
342		min	-2182.577	3	-6.608	4	0	1	0	1	0	1	0	1
343	M8	1	max	3128	2	0	1	0	1	0	1	0	1	1
344		min	-923.596	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3128.17	2	0	1	0	1	0	1	0	1	0	1
346		min	-923.469	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3128.34	2	0	1	0	1	0	1	0	1	0	1
348		min	-923.341	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3128.511	2	0	1	0	1	0	1	0	1	0	1
350		min	-923.213	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3128.681	2	0	1	0	1	0	1	0	1	0	1
352		min	-923.085	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3128.851	2	0	1	0	1	0	1	0	1	0	1
354		min	-922.958	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3129.022	2	0	1	0	1	0	1	0	1	0	1
356		min	-922.83	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3129.192	2	0	1	0	1	0	1	0	1	0	1
358		min	-922.702	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3129.362	2	0	1	0	1	0	1	0	1	0	1
360		min	-922.574	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3129.533	2	0	1	0	1	0	1	0	1	0	1
362		min	-922.447	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3129.703	2	0	1	0	1	0	1	0	1	0	1
364		min	-922.319	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3129.873	2	0	1	0	1	0	1	0	1	0	1
366		min	-922.191	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3130.044	2	0	1	0	1	0	1	0	1	0	1
368		min	-922.063	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3130.214	2	0	1	0	1	0	1	0	1	0	1
370			min	-921.936	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3130.384	2	0	1	0	1	0	1	0	1	0	1
372			min	-921.808	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3130.555	2	0	1	0	1	0	1	0	1	0	1
374			min	-921.68	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3130.725	2	0	1	0	1	0	1	0	1	0	1
376			min	-921.552	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3130.895	2	0	1	0	1	0	1	0	1	0	1
378			min	-921.424	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3131.066	2	0	1	0	1	0	1	0	1	0	1
380			min	-921.297	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1089.048	2	2.158	4	-.02	15	0	1	0	1	0	1
382			min	-1427.791	3	.507	15	-.551	1	0	3	0	3	0	1
383		2	max	1089.464	2	2.149	4	-.02	15	0	1	0	15	0	15
384			min	-1427.479	3	.505	15	-.551	1	0	3	0	1	0	4
385		3	max	1089.88	2	2.141	4	-.02	15	0	1	0	15	0	15
386			min	-1427.167	3	.503	15	-.551	1	0	3	0	1	-.001	4
387		4	max	1090.296	2	2.132	4	-.02	15	0	1	0	15	0	15
388			min	-1426.855	3	.501	15	-.551	1	0	3	0	1	-.002	4
389		5	max	1090.712	2	2.123	4	-.02	15	0	1	0	15	0	15
390			min	-1426.543	3	.499	15	-.551	1	0	3	0	1	-.002	4
391		6	max	1091.127	2	2.114	4	-.02	15	0	1	0	15	0	15
392			min	-1426.231	3	.497	15	-.551	1	0	3	0	1	-.003	4
393		7	max	1091.543	2	2.106	4	-.02	15	0	1	0	15	0	15
394			min	-1425.92	3	.495	15	-.551	1	0	3	0	1	-.004	4
395		8	max	1091.959	2	2.097	4	-.02	15	0	1	0	15	0	15
396			min	-1425.608	3	.493	15	-.551	1	0	3	-.001	1	-.004	4
397		9	max	1092.375	2	2.088	4	-.02	15	0	1	0	15	-.001	15
398			min	-1425.296	3	.491	15	-.551	1	0	3	-.001	1	-.005	4
399		10	max	1092.791	2	2.08	4	-.02	15	0	1	0	15	-.001	15
400			min	-1424.984	3	.489	15	-.551	1	0	3	-.001	1	-.005	4
401		11	max	1093.207	2	2.071	4	-.02	15	0	1	0	15	-.001	15
402			min	-1424.672	3	.487	15	-.551	1	0	3	-.002	1	-.006	4
403		12	max	1093.623	2	2.062	4	-.02	15	0	1	0	15	-.002	15
404			min	-1424.36	3	.485	15	-.551	1	0	3	-.002	1	-.007	4
405		13	max	1094.039	2	2.053	4	-.02	15	0	1	0	15	-.002	15
406			min	-1424.048	3	.483	15	-.551	1	0	3	-.002	1	-.007	4
407		14	max	1094.455	2	2.045	4	-.02	15	0	1	0	15	-.002	15
408			min	-1423.736	3	.481	15	-.551	1	0	3	-.002	1	-.008	4
409		15	max	1094.87	2	2.036	4	-.02	15	0	1	0	15	-.002	15
410			min	-1423.424	3	.479	15	-.551	1	0	3	-.002	1	-.008	4
411		16	max	1095.286	2	2.027	4	-.02	15	0	1	0	15	-.002	15
412			min	-1423.112	3	.476	15	-.551	1	0	3	-.002	1	-.009	4
413		17	max	1095.702	2	2.019	4	-.02	15	0	1	0	15	-.002	15
414			min	-1422.801	3	.474	15	-.551	1	0	3	-.002	1	-.009	4
415		18	max	1096.118	2	2.01	4	-.02	15	0	1	0	15	-.002	15
416			min	-1422.489	3	.472	15	-.551	1	0	3	-.003	1	-.01	4
417		19	max	1096.534	2	2.001	4	-.02	15	0	1	0	15	-.002	15
418			min	-1422.177	3	.47	15	-.551	1	0	3	-.003	1	-.01	4
419	M11	1	max	600.629	2	9.101	4	-.005	15	0	1	0	15	.01	4
420			min	-738.36	3	2.139	15	-.137	1	0	3	0	1	.002	15
421		2	max	600.458	2	8.227	4	-.005	15	0	1	0	15	.006	2
422			min	-738.488	3	1.934	15	-.137	1	0	3	0	1	.001	12
423		3	max	600.288	2	7.353	4	-.005	15	0	1	0	15	.003	2
424			min	-738.616	3	1.728	15	-.137	1	0	3	0	1	0	3
425		4	max	600.118	2	6.478	4	-.005	15	0	1	0	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-738.744	3	1.523	15	-.137	1	0	3	0	1	-.002	3
427		5	max	599.947	2	5.604	4	-.005	15	0	1	0	15	0	15
428			min	-738.871	3	1.317	15	-.137	1	0	3	0	1	-.004	3
429		6	max	599.777	2	4.729	4	-.005	15	0	1	0	15	-.001	15
430			min	-738.999	3	1.112	15	-.137	1	0	3	0	1	-.006	4
431		7	max	599.606	2	3.855	4	-.005	15	0	1	0	15	-.002	15
432			min	-739.127	3	.906	15	-.137	1	0	3	0	1	-.008	4
433		8	max	599.436	2	2.98	4	-.005	15	0	1	0	15	-.002	15
434			min	-739.255	3	.701	15	-.137	1	0	3	0	1	-.01	4
435		9	max	599.266	2	2.106	4	-.005	15	0	1	0	15	-.003	15
436			min	-739.382	3	.495	15	-.137	1	0	3	0	1	-.011	4
437		10	max	599.095	2	1.231	4	-.005	15	0	1	0	15	-.003	15
438			min	-739.51	3	.289	15	-.137	1	0	3	0	1	-.012	4
439		11	max	598.925	2	.455	2	-.005	15	0	1	0	15	-.003	15
440			min	-739.638	3	-.035	3	-.137	1	0	3	0	1	-.012	4
441		12	max	598.755	2	-.122	15	-.005	15	0	1	0	15	-.003	15
442			min	-739.766	3	-.546	3	-.137	1	0	3	0	1	-.012	4
443		13	max	598.584	2	-.327	15	-.005	15	0	1	0	15	-.003	15
444			min	-739.893	3	-1.392	4	-.137	1	0	3	0	1	-.011	4
445		14	max	598.414	2	-.533	15	-.005	15	0	1	0	15	-.002	15
446			min	-740.021	3	-2.266	4	-.137	1	0	3	0	1	-.011	4
447		15	max	598.244	2	-.738	15	-.005	15	0	1	0	15	-.002	15
448			min	-740.149	3	-3.141	4	-.137	1	0	3	0	1	-.009	4
449		16	max	598.073	2	-.944	15	-.005	15	0	1	0	15	-.002	15
450			min	-740.277	3	-4.015	4	-.137	1	0	3	-.001	1	-.008	4
451		17	max	597.903	2	-1.149	15	-.005	15	0	1	0	15	-.001	15
452			min	-740.404	3	-4.89	4	-.137	1	0	3	-.001	1	-.005	4
453		18	max	597.733	2	-1.355	15	-.005	15	0	1	0	15	0	15
454			min	-740.532	3	-5.764	4	-.137	1	0	3	-.001	1	-.003	4
455		19	max	597.562	2	-1.561	15	-.005	15	0	1	0	15	0	1
456			min	-740.66	3	-6.638	4	-.137	1	0	3	-.001	1	0	1
457	M12	1	max	1124.511	1	0	1	7.851	1	0	1	0	15	0	1
458			min	-280.18	3	0	1	.285	15	0	1	0	1	0	1
459		2	max	1124.681	1	0	1	7.851	1	0	1	0	1	0	1
460			min	-280.052	3	0	1	.285	15	0	1	0	12	0	1
461		3	max	1124.852	1	0	1	7.851	1	0	1	.001	1	0	1
462			min	-279.924	3	0	1	.285	15	0	1	0	15	0	1
463		4	max	1125.022	1	0	1	7.851	1	0	1	.002	1	0	1
464			min	-279.797	3	0	1	.285	15	0	1	0	15	0	1
465		5	max	1125.192	1	0	1	7.851	1	0	1	.003	1	0	1
466			min	-279.669	3	0	1	.285	15	0	1	0	15	0	1
467		6	max	1125.363	1	0	1	7.851	1	0	1	.004	1	0	1
468			min	-279.541	3	0	1	.285	15	0	1	0	15	0	1
469		7	max	1125.533	1	0	1	7.851	1	0	1	.005	1	0	1
470			min	-279.413	3	0	1	.285	15	0	1	0	15	0	1
471		8	max	1125.703	1	0	1	7.851	1	0	1	.006	1	0	1
472			min	-279.286	3	0	1	.285	15	0	1	0	15	0	1
473		9	max	1125.874	1	0	1	7.851	1	0	1	.006	1	0	1
474			min	-279.158	3	0	1	.285	15	0	1	0	15	0	1
475		10	max	1126.044	1	0	1	7.851	1	0	1	.007	1	0	1
476			min	-279.03	3	0	1	.285	15	0	1	0	15	0	1
477		11	max	1126.214	1	0	1	7.851	1	0	1	.008	1	0	1
478			min	-278.902	3	0	1	.285	15	0	1	0	15	0	1
479		12	max	1126.385	1	0	1	7.851	1	0	1	.009	1	0	1
480			min	-278.775	3	0	1	.285	15	0	1	0	15	0	1
481		13	max	1126.555	1	0	1	7.851	1	0	1	.01	1	0	1
482			min	-278.647	3	0	1	.285	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1126.725	1	0	1	7.851	1	0	1	.011	1	0	1
484		min	-278.519	3	0	1	.285	15	0	1	0	15	0	1
485	15	max	1126.896	1	0	1	7.851	1	0	1	.012	1	0	1
486		min	-278.391	3	0	1	.285	15	0	1	0	15	0	1
487	16	max	1127.066	1	0	1	7.851	1	0	1	.013	1	0	1
488		min	-278.264	3	0	1	.285	15	0	1	0	15	0	1
489	17	max	1127.236	1	0	1	7.851	1	0	1	.014	1	0	1
490		min	-278.136	3	0	1	.285	15	0	1	0	15	0	1
491	18	max	1127.407	1	0	1	7.851	1	0	1	.015	1	0	1
492		min	-278.008	3	0	1	.285	15	0	1	0	15	0	1
493	19	max	1127.577	1	0	1	7.851	1	0	1	.016	1	0	1
494		min	-277.88	3	0	1	.285	15	0	1	0	15	0	1
495	M1	1	max	133.892	1	763.905	3	-1.72	15	0	.131	1	0	15
496		min	4.807	15	-450.653	1	-46.926	1	0	3	.005	15	-.014	2
497	2	max	134.468	1	762.718	3	-1.72	15	0	1	.102	1	.268	1
498		min	4.981	15	-452.236	1	-46.926	1	0	3	.004	15	-.48	3
499	3	max	478.071	3	580.212	2	-1.7	15	0	3	.072	1	.538	1
500		min	-301.822	2	-592.827	3	-46.508	1	0	2	.003	15	-.938	3
501	4	max	478.504	3	578.629	2	-1.7	15	0	3	.044	1	.189	1
502		min	-301.246	2	-594.015	3	-46.508	1	0	2	.002	15	-.57	3
503	5	max	478.936	3	577.045	2	-1.7	15	0	3	.015	1	-.006	15
504		min	-300.67	2	-595.202	3	-46.508	1	0	2	0	15	-.201	3
505	6	max	479.368	3	575.462	2	-1.7	15	0	3	0	15	.169	3
506		min	-300.093	2	-596.39	3	-46.508	1	0	2	-.014	1	-.552	2
507	7	max	479.8	3	573.879	2	-1.7	15	0	3	-.002	15	.539	3
508		min	-299.517	2	-597.577	3	-46.508	1	0	2	-.043	1	-.908	2
509	8	max	480.232	3	572.296	2	-1.7	15	0	3	-.003	15	.91	3
510		min	-298.941	2	-598.764	3	-46.508	1	0	2	-.072	1	-1.264	2
511	9	max	491.414	3	48.238	2	-2.843	15	0	9	.048	1	1.06	3
512		min	-245.049	2	.482	15	-77.879	1	0	3	.002	15	-1.441	2
513	10	max	491.847	3	46.655	2	-2.843	15	0	9	0	10	1.038	3
514		min	-244.473	2	.004	15	-77.879	1	0	3	0	1	-1.47	2
515	11	max	492.279	3	45.072	2	-2.843	15	0	9	-.002	15	1.016	3
516		min	-243.896	2	-1.948	4	-77.879	1	0	3	-.049	1	-1.499	2
517	12	max	503.198	3	401.058	3	-1.638	15	0	2	.071	1	.891	3
518		min	-189.887	2	-674.819	2	-45.072	1	0	3	.003	15	-1.33	2
519	13	max	503.63	3	399.871	3	-1.638	15	0	2	.043	1	.643	3
520		min	-189.311	2	-676.402	2	-45.072	1	0	3	.002	15	-.911	2
521	14	max	504.063	3	398.683	3	-1.638	15	0	2	.015	1	.395	3
522		min	-188.735	2	-677.985	2	-45.072	1	0	3	0	15	-.491	2
523	15	max	504.495	3	397.496	3	-1.638	15	0	2	0	15	.148	3
524		min	-188.159	2	-679.568	2	-45.072	1	0	3	-.013	1	-.097	1
525	16	max	504.927	3	396.308	3	-1.638	15	0	2	-.001	15	.353	2
526		min	-187.582	2	-681.152	2	-45.072	1	0	3	-.041	1	-.098	3
527	17	max	505.359	3	395.121	3	-1.638	15	0	2	-.003	15	.776	2
528		min	-187.006	2	-682.735	2	-45.072	1	0	3	-.069	1	-.344	3
529	18	max	-4.994	15	642.325	2	-1.918	15	0	3	-.004	15	.391	2
530		min	-135.049	1	-293.083	3	-52.559	1	0	2	-.1	1	-.17	3
531	19	max	-4.82	15	640.742	2	-1.918	15	0	3	-.005	15	.012	3
532		min	-134.472	1	-294.271	3	-52.559	1	0	2	-.133	1	-.008	1
533	M5	1	max	304.124	1	2516.935	3	0	1	0	0	1	.028	2
534		min	7.178	12	-1562.253	1	0	1	0	1	0	1	0	15
535	2	max	304.7	1	2515.748	3	0	1	0	1	0	1	.993	1
536		min	7.467	12	-1563.836	1	0	1	0	1	0	1	-1.549	3
537	3	max	1452.371	3	1512.621	2	0	1	0	1	0	1	1.931	1
538		min	-942.33	2	-1698.014	3	0	1	0	1	0	1	-3.064	3
539	4	max	1452.803	3	1511.037	2	0	1	0	1	0	1	1.012	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-941.754	2	-1699.201	3	0	1	0	1	0	1	-2.009	3
541		5	max	1453.235	3	1509.454	2	0	1	0	1	0	1	.093	1
542			min	-941.178	2	-1700.389	3	0	1	0	1	0	1	-.955	3
543		6	max	1453.667	3	1507.871	2	0	1	0	1	0	1	.101	3
544			min	-940.601	2	-1701.576	3	0	1	0	1	0	1	-.906	2
545		7	max	1454.1	3	1506.288	2	0	1	0	1	0	1	1.158	3
546			min	-940.025	2	-1702.764	3	0	1	0	1	0	1	-1.841	2
547		8	max	1454.532	3	1504.705	2	0	1	0	1	0	1	2.215	3
548			min	-939.449	2	-1703.951	3	0	1	0	1	0	1	-2.775	2
549		9	max	1464.521	3	164.303	2	0	1	0	1	0	1	2.556	3
550			min	-820.357	2	.477	15	0	1	0	1	0	1	-3.178	2
551		10	max	1464.953	3	162.72	2	0	1	0	1	0	1	2.466	3
552			min	-819.781	2	0	15	0	1	0	1	0	1	-3.279	2
553		11	max	1465.385	3	161.137	2	0	1	0	1	0	1	2.377	3
554			min	-819.205	2	-1.854	4	0	1	0	1	0	1	-3.38	2
555		12	max	1475.899	3	1091.202	3	0	1	0	1	0	1	2.077	3
556			min	-700.347	2	-1831.807	2	0	1	0	1	0	1	-3.018	2
557		13	max	1476.331	3	1090.015	3	0	1	0	1	0	1	1.4	3
558			min	-699.771	2	-1833.391	2	0	1	0	1	0	1	-1.881	2
559		14	max	1476.763	3	1088.828	3	0	1	0	1	0	1	.724	3
560			min	-699.195	2	-1834.974	2	0	1	0	1	0	1	-.742	2
561		15	max	1477.196	3	1087.64	3	0	1	0	1	0	1	.397	2
562			min	-698.619	2	-1836.557	2	0	1	0	1	0	1	0	15
563		16	max	1477.628	3	1086.453	3	0	1	0	1	0	1	1.537	2
564			min	-698.042	2	-1838.14	2	0	1	0	1	0	1	-.626	3
565		17	max	1478.06	3	1085.266	3	0	1	0	1	0	1	2.679	2
566			min	-697.466	2	-1839.723	2	0	1	0	1	0	1	-1.3	3
567		18	max	-8.506	12	2182.209	2	0	1	0	1	0	1	1.368	2
568			min	-303.54	1	-1046.349	3	0	1	0	1	0	1	-.675	3
569		19	max	-8.218	12	2180.626	2	0	1	0	1	0	1	.017	1
570			min	-302.963	1	-1047.536	3	0	1	0	1	0	1	-.025	3
571	M9	1	max	133.892	1	763.905	3	46.926	1	0	3	-.005	15	0	15
572			min	4.807	15	-450.653	1	1.72	15	0	1	-.131	1	-.014	2
573		2	max	134.468	1	762.718	3	46.926	1	0	3	-.004	15	.268	1
574			min	4.981	15	-452.236	1	1.72	15	0	1	-.102	1	-.48	3
575		3	max	478.071	3	580.212	2	46.508	1	0	2	-.003	15	.538	1
576			min	-301.822	2	-592.827	3	1.7	15	0	3	-.072	1	-.938	3
577		4	max	478.504	3	578.629	2	46.508	1	0	2	-.002	15	.189	1
578			min	-301.246	2	-594.015	3	1.7	15	0	3	-.044	1	-.57	3
579		5	max	478.936	3	577.045	2	46.508	1	0	2	0	15	-.006	15
580			min	-300.67	2	-595.202	3	1.7	15	0	3	-.015	1	-.201	3
581		6	max	479.368	3	575.462	2	46.508	1	0	2	.014	1	.169	3
582			min	-300.093	2	-596.39	3	1.7	15	0	3	0	15	-.552	2
583		7	max	479.8	3	573.879	2	46.508	1	0	2	.043	1	.539	3
584			min	-299.517	2	-597.577	3	1.7	15	0	3	.002	15	-.908	2
585		8	max	480.232	3	572.296	2	46.508	1	0	2	.072	1	.91	3
586			min	-298.941	2	-598.764	3	1.7	15	0	3	.003	15	-1.264	2
587		9	max	491.414	3	48.238	2	77.879	1	0	3	-.002	15	1.06	3
588			min	-245.049	2	.482	15	2.843	15	0	9	-.048	1	-1.441	2
589		10	max	491.847	3	46.655	2	77.879	1	0	3	0	1	1.038	3
590			min	-244.473	2	.004	15	2.843	15	0	9	0	10	-1.47	2
591		11	max	492.279	3	45.072	2	77.879	1	0	3	.049	1	1.016	3
592			min	-243.896	2	-1.948	4	2.843	15	0	9	.002	15	-1.499	2
593		12	max	503.198	3	401.058	3	45.072	1	0	3	-.003	15	.891	3
594			min	-189.887	2	-674.819	2	1.638	15	0	2	-.071	1	-1.33	2
595		13	max	503.63	3	399.871	3	45.072	1	0	3	-.002	15	.643	3
596			min	-189.311	2	-676.402	2	1.638	15	0	2	-.043	1	-.911	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	504.063	3	398.683	3	45.072	1	0	3	0	15	.395	3
598		min	-188.735	2	-677.985	2	1.638	15	0	2	-.015	1	-.491	2
599	15	max	504.495	3	397.496	3	45.072	1	0	3	.013	1	.148	3
600		min	-188.159	2	-679.568	2	1.638	15	0	2	0	15	-.097	1
601	16	max	504.927	3	396.308	3	45.072	1	0	3	.041	1	.353	2
602		min	-187.582	2	-681.152	2	1.638	15	0	2	.001	15	-.098	3
603	17	max	505.359	3	395.121	3	45.072	1	0	3	.069	1	.776	2
604		min	-187.006	2	-682.735	2	1.638	15	0	2	.003	15	-.344	3
605	18	max	-4.994	15	642.325	2	52.559	1	0	2	.1	1	.391	2
606		min	-135.049	1	-293.083	3	1.918	15	0	3	.004	15	-.17	3
607	19	max	-4.82	15	640.742	2	52.559	1	0	2	.133	1	.012	3
608		min	-134.472	1	-294.271	3	1.918	15	0	3	.005	15	-.008	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.229	2	.009	3	1.553e-2	2	NC	1	NC	1
2			min	0	15	-.067	3	-.006	2	-4.291e-3	3	NC	1	NC	1
3		2	max	0	1	.169	2	.013	1	1.65e-2	2	NC	4	NC	1
4			min	0	15	.004	15	-.003	10	-3.832e-3	3	1195.936	3	NC	1
5		3	max	0	1	.198	3	.03	1	1.748e-2	2	NC	5	NC	2
6			min	0	15	.003	15	-.002	10	-3.373e-3	3	657.267	3	5629.205	1
7		4	max	0	1	.273	3	.044	1	1.845e-2	2	NC	5	NC	2
8			min	0	15	.002	15	-.002	10	-2.914e-3	3	511.747	3	3863.43	1
9		5	max	0	1	.296	3	.051	1	1.943e-2	2	NC	5	NC	2
10			min	0	15	.003	15	-.002	10	-2.455e-3	3	479.415	3	3388.815	1
11		6	max	0	1	.267	3	.047	1	2.04e-2	2	NC	5	NC	2
12			min	0	15	.003	15	-.004	10	-1.996e-3	3	520.24	3	3629.342	1
13		7	max	0	1	.212	2	.035	1	2.138e-2	2	NC	2	NC	2
14			min	0	15	.005	15	-.006	10	-1.538e-3	3	657.042	3	4889.446	1
15		8	max	0	1	.28	2	.027	3	2.235e-2	2	NC	4	NC	2
16			min	0	15	.006	15	-.009	10	-1.079e-3	3	1002.255	3	9839.348	1
17		9	max	0	1	.339	2	.027	3	2.333e-2	2	NC	4	NC	1
18			min	0	15	.007	15	-.015	2	-6.197e-4	3	1583.044	2	9625.824	3
19		10	max	0	1	.365	2	.028	3	2.431e-2	2	NC	4	NC	1
20		min	0	1	-.016	3	-.019	2	-1.608e-4	3	1280.245	2	9556.644	3	
21	11	max	0	15	.339	2	.027	3	2.333e-2	2	NC	4	NC	1	
22		min	0	1	.007	15	-.015	2	-6.197e-4	3	1583.044	2	9625.824	3	
23	12	max	0	15	.28	2	.027	3	2.235e-2	2	NC	4	NC	2	
24		min	0	1	.006	15	-.009	10	-1.079e-3	3	1002.255	3	9839.348	1	
25	13	max	0	15	.212	2	.035	1	2.138e-2	2	NC	2	NC	2	
26		min	0	1	.005	15	-.006	10	-1.538e-3	3	657.042	3	4889.446	1	
27	14	max	0	15	.267	3	.047	1	2.04e-2	2	NC	5	NC	2	
28		min	0	1	.003	15	-.004	10	-1.996e-3	3	520.24	3	3629.342	1	
29	15	max	0	15	.296	3	.051	1	1.943e-2	2	NC	5	NC	2	
30		min	0	1	.003	15	-.002	10	-2.455e-3	3	479.415	3	3388.815	1	
31	16	max	0	15	.273	3	.044	1	1.845e-2	2	NC	5	NC	2	
32		min	0	1	.002	15	-.002	10	-2.914e-3	3	511.747	3	3863.43	1	
33	17	max	0	15	.198	3	.03	1	1.748e-2	2	NC	5	NC	2	
34		min	0	1	.003	15	-.002	10	-3.373e-3	3	657.267	3	5629.205	1	
35	18	max	0	15	.169	2	.013	1	1.65e-2	2	NC	4	NC	1	
36		min	0	1	.004	15	-.003	10	-3.832e-3	3	1195.936	3	NC	1	
37	19	max	0	15	.229	2	.009	3	1.553e-2	2	NC	1	NC	1	
38		min	0	1	-.067	3	-.006	2	-4.291e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.443	3	.008	3	8.573e-3	2	NC	1	NC	1
40			min	0	15	-.667	2	-.005	2	-6.675e-3	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.637	3	.009	3	9.742e-3	2	NC	5	NC	1
42			min	0	15	-.871	2	-.003	10	-7.705e-3	3	855.3	2	NC	1
43		3	max	0	1	.81	3	.023	1	1.091e-2	2	NC	5	NC	2
44			min	0	15	-1.056	2	-.002	10	-8.735e-3	3	448.159	2	7576.632	1
45		4	max	0	1	.944	3	.036	1	1.208e-2	2	NC	15	NC	2
46			min	0	15	-1.208	2	-.002	10	-9.765e-3	3	321.653	2	4780.15	1
47		5	max	0	1	1.033	3	.043	1	1.325e-2	2	NC	15	NC	2
48			min	0	15	-1.321	2	-.002	10	-1.079e-2	3	266.204	2	3997.89	1
49		6	max	0	1	1.074	3	.042	1	1.442e-2	2	NC	15	NC	2
50			min	0	15	-1.391	2	-.003	10	-1.182e-2	3	240.34	2	4149.558	1
51		7	max	0	1	1.074	3	.031	1	1.559e-2	2	NC	15	NC	2
52			min	0	15	-1.423	2	-.005	10	-1.285e-2	3	230.388	2	5458.654	1
53		8	max	0	1	1.044	3	.024	3	1.676e-2	2	NC	15	NC	1
54			min	0	15	-1.424	2	-.008	10	-1.388e-2	3	230.062	2	NC	1
55		9	max	0	1	1.006	3	.024	3	1.793e-2	2	NC	15	NC	1
56			min	0	15	-1.409	2	-.014	2	-1.491e-2	3	234.616	2	NC	1
57		10	max	0	1	.986	3	.024	3	1.91e-2	2	NC	15	NC	1
58			min	0	1	-1.399	2	-.017	2	-1.594e-2	3	237.948	2	NC	1
59		11	max	0	15	1.006	3	.024	3	1.793e-2	2	NC	15	NC	1
60			min	0	1	-1.409	2	-.014	2	-1.491e-2	3	234.616	2	NC	1
61		12	max	0	15	1.044	3	.024	3	1.676e-2	2	NC	15	NC	1
62			min	0	1	-1.424	2	-.008	10	-1.388e-2	3	230.062	2	NC	1
63		13	max	0	15	1.074	3	.031	1	1.559e-2	2	NC	15	NC	2
64			min	0	1	-1.423	2	-.005	10	-1.285e-2	3	230.388	2	5458.654	1
65		14	max	0	15	1.074	3	.042	1	1.442e-2	2	NC	15	NC	2
66			min	0	1	-1.391	2	-.003	10	-1.182e-2	3	240.34	2	4149.558	1
67		15	max	0	15	1.033	3	.043	1	1.325e-2	2	NC	15	NC	2
68			min	0	1	-1.321	2	-.002	10	-1.079e-2	3	266.204	2	3997.89	1
69		16	max	0	15	.944	3	.036	1	1.208e-2	2	NC	15	NC	2
70			min	0	1	-1.208	2	-.002	10	-9.765e-3	3	321.653	2	4780.15	1
71		17	max	0	15	.81	3	.023	1	1.091e-2	2	NC	5	NC	2
72			min	0	1	-1.056	2	-.002	10	-8.735e-3	3	448.159	2	7576.632	1
73		18	max	0	15	.637	3	.009	3	9.742e-3	2	NC	5	NC	1
74			min	0	1	-.871	2	-.003	10	-7.705e-3	3	855.3	2	NC	1
75		19	max	0	15	.443	3	.008	3	8.573e-3	2	NC	1	NC	1
76			min	0	1	-.667	2	-.005	2	-6.675e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.453	3	.008	3	5.642e-3	3	NC	1	NC	1
78			min	0	1	-.666	2	-.005	2	-8.869e-3	2	NC	1	NC	1
79		2	max	0	15	.601	3	.009	3	6.495e-3	3	NC	5	NC	1
80			min	0	1	-.905	2	-.003	10	-1.008e-2	2	729.398	2	NC	1
81		3	max	0	15	.736	3	.023	1	7.348e-3	3	NC	5	NC	2
82			min	0	1	-1.118	2	-.002	10	-1.13e-2	2	385.097	2	7520.984	1
83		4	max	0	15	.849	3	.036	1	8.201e-3	3	NC	15	NC	2
84			min	0	1	-1.288	2	-.001	10	-1.252e-2	2	279.798	2	4748.094	1
85		5	max	0	15	.934	3	.044	1	9.054e-3	3	NC	15	NC	2
86			min	0	1	-1.405	2	-.002	10	-1.373e-2	2	235.553	2	3969.282	1
87		6	max	0	15	.989	3	.042	1	9.907e-3	3	NC	15	NC	2
88			min	0	1	-1.466	2	-.003	10	-1.495e-2	2	217.437	2	4113.037	1
89		7	max	0	15	1.017	3	.032	1	1.076e-2	3	NC	15	NC	2
90			min	0	1	-1.479	2	-.005	10	-1.616e-2	2	214.165	2	5387.612	1
91		8	max	0	15	1.022	3	.022	3	1.161e-2	3	NC	15	NC	1
92			min	0	1	-1.455	2	-.007	10	-1.738e-2	2	220.457	2	NC	1
93		9	max	0	15	1.016	3	.023	3	1.247e-2	3	NC	15	NC	1
94			min	0	1	-1.419	2	-.013	2	-1.859e-2	2	231.212	2	NC	1
95		10	max	0	1	1.01	3	.023	3	1.332e-2	3	NC	15	NC	1
96			min	0	1	-1.398	2	-.016	2	-1.981e-2	2	237.653	2	NC	1
97		11	max	0	1	1.016	3	.023	3	1.247e-2	3	NC	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.419	2	-.013	2	-1.859e-2	2	231.212	2	NC	1
99		max	0	1	1.022	3	.022	3	1.161e-2	3	NC	15	NC	1
100		min	0	15	-1.455	2	-.007	10	-1.738e-2	2	220.457	2	NC	1
101		max	0	1	1.017	3	.032	1	1.076e-2	3	NC	15	NC	2
102		min	0	15	-1.479	2	-.005	10	-1.616e-2	2	214.165	2	5387.612	1
103		max	0	1	.989	3	.042	1	9.907e-3	3	NC	15	NC	2
104		min	0	15	-1.466	2	-.003	10	-1.495e-2	2	217.437	2	4113.037	1
105		max	0	1	.934	3	.044	1	9.054e-3	3	NC	15	NC	2
106		min	0	15	-1.405	2	-.002	10	-1.373e-2	2	235.553	2	3969.282	1
107		max	0	1	.849	3	.036	1	8.201e-3	3	NC	15	NC	2
108		min	0	15	-1.288	2	-.001	10	-1.252e-2	2	279.798	2	4748.094	1
109		max	0	1	.736	3	.023	1	7.348e-3	3	NC	5	NC	2
110		min	0	15	-1.118	2	-.002	10	-1.13e-2	2	385.097	2	7520.984	1
111		max	0	1	.601	3	.009	3	6.495e-3	3	NC	5	NC	1
112		min	0	15	-.905	2	-.003	10	-1.008e-2	2	729.398	2	NC	1
113		max	0	1	.453	3	.008	3	5.642e-3	3	NC	1	NC	1
114		min	0	15	-.666	2	-.005	2	-8.869e-3	2	NC	1	NC	1
115	M16	max	0	15	.204	2	.007	3	1.083e-2	3	NC	1	NC	1
116		min	0	1	-.162	3	-.004	2	-1.325e-2	2	NC	1	NC	1
117		max	0	15	.115	1	.013	1	1.172e-2	3	NC	4	NC	1
118		min	0	1	-.128	3	-.002	10	-1.374e-2	2	1698.857	2	NC	1
119		max	0	15	.049	1	.031	1	1.26e-2	3	NC	5	NC	2
120		min	0	1	-.104	3	0	10	-1.423e-2	2	951.115	2	5621.742	1
121		max	0	15	.024	9	.045	1	1.348e-2	3	NC	5	NC	2
122		min	0	1	-.096	3	0	10	-1.48e-2	1	766.626	2	3837.742	1
123		max	0	15	.026	9	.052	1	1.436e-2	3	NC	5	NC	2
124		min	0	1	-.108	3	0	10	-1.539e-2	1	763.586	2	3346.034	1
125		max	0	15	.055	1	.049	1	1.525e-2	3	NC	4	NC	2
126		min	0	1	-.138	3	-.002	10	-1.598e-2	1	930.496	2	3551.047	1
127		max	0	15	.121	1	.037	1	1.613e-2	3	NC	4	NC	2
128		min	0	1	-.182	3	-.004	10	-1.656e-2	1	1530.934	2	4697.488	1
129		max	0	15	.2	1	.019	3	1.701e-2	3	NC	1	NC	2
130		min	0	1	-.231	3	-.006	10	-1.715e-2	1	2540.393	3	8930.85	1
131		max	0	15	.269	1	.02	3	1.789e-2	3	NC	4	NC	1
132		min	0	1	-.272	3	-.011	2	-1.774e-2	1	1581.927	3	NC	1
133		max	0	1	.299	1	.02	3	1.878e-2	3	NC	5	NC	1
134		min	0	1	-.29	3	-.015	2	-1.833e-2	1	1357.465	3	NC	1
135		max	0	1	.269	1	.02	3	1.789e-2	3	NC	4	NC	1
136		min	0	15	-.272	3	-.011	2	-1.774e-2	1	1581.927	3	NC	1
137		max	0	1	.2	1	.019	3	1.701e-2	3	NC	1	NC	2
138		min	0	15	-.231	3	-.006	10	-1.715e-2	1	2540.393	3	8930.85	1
139		max	0	1	.121	1	.037	1	1.613e-2	3	NC	4	NC	2
140		min	0	15	-.182	3	-.004	10	-1.656e-2	1	1530.934	2	4697.488	1
141		max	0	1	.055	1	.049	1	1.525e-2	3	NC	4	NC	2
142		min	0	15	-.138	3	-.002	10	-1.598e-2	1	930.496	2	3551.047	1
143		max	0	1	.026	9	.052	1	1.436e-2	3	NC	5	NC	2
144		min	0	15	-.108	3	0	10	-1.539e-2	1	763.586	2	3346.034	1
145		max	0	1	.024	9	.045	1	1.348e-2	3	NC	5	NC	2
146		min	0	15	-.096	3	0	10	-1.48e-2	1	766.626	2	3837.742	1
147		max	0	1	.049	1	.031	1	1.26e-2	3	NC	5	NC	2
148		min	0	15	-.104	3	0	10	-1.423e-2	2	951.115	2	5621.742	1
149		max	0	1	.115	1	.013	1	1.172e-2	3	NC	4	NC	1
150		min	0	15	-.128	3	-.002	10	-1.374e-2	2	1698.857	2	NC	1
151		max	0	1	.204	2	.007	3	1.083e-2	3	NC	1	NC	1
152		min	0	15	-.162	3	-.004	2	-1.325e-2	2	NC	1	NC	1
153	M2	max	.006	2	.008	2	.006	1	-4.817e-6	15	NC	1	NC	2
154		min	-.008	3	-.013	3	0	15	-1.315e-4	1	7464.909	2	9971.616	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155	2	max	.006	2	.007	2	.006	1	-4.515e-6	15	NC	1	NC	1
156		min	-.008	3	-.012	3	0	15	-1.233e-4	1	8563.978	2	NC	1
157	3	max	.006	2	.006	2	.005	1	-4.213e-6	15	NC	1	NC	1
158		min	-.007	3	-.012	3	0	15	-1.15e-4	1	NC	1	NC	1
159	4	max	.005	2	.005	2	.005	1	-3.911e-6	15	NC	1	NC	1
160		min	-.007	3	-.011	3	0	15	-1.067e-4	1	NC	1	NC	1
161	5	max	.005	2	.004	2	.004	1	-3.609e-6	15	NC	1	NC	1
162		min	-.006	3	-.011	3	0	15	-9.845e-5	1	NC	1	NC	1
163	6	max	.005	2	.003	2	.004	1	-3.307e-6	15	NC	1	NC	1
164		min	-.006	3	-.01	3	0	15	-9.018e-5	1	NC	1	NC	1
165	7	max	.004	2	.002	2	.003	1	-3.005e-6	15	NC	1	NC	1
166		min	-.006	3	-.01	3	0	15	-8.191e-5	1	NC	1	NC	1
167	8	max	.004	2	.001	2	.003	1	-2.703e-6	15	NC	1	NC	1
168		min	-.005	3	-.009	3	0	15	-7.364e-5	1	NC	1	NC	1
169	9	max	.004	2	0	2	.002	1	-2.401e-6	15	NC	1	NC	1
170		min	-.005	3	-.009	3	0	15	-6.537e-5	1	NC	1	NC	1
171	10	max	.003	2	0	2	.002	1	-2.099e-6	15	NC	1	NC	1
172		min	-.004	3	-.008	3	0	15	-5.71e-5	1	NC	1	NC	1
173	11	max	.003	2	0	2	.002	1	-1.797e-6	15	NC	1	NC	1
174		min	-.004	3	-.007	3	0	15	-4.883e-5	1	NC	1	NC	1
175	12	max	.002	2	0	2	.001	1	-1.496e-6	15	NC	1	NC	1
176		min	-.003	3	-.007	3	0	15	-4.056e-5	1	NC	1	NC	1
177	13	max	.002	2	-.001	15	0	1	-1.194e-6	15	NC	1	NC	1
178		min	-.003	3	-.006	3	0	15	-3.229e-5	1	NC	1	NC	1
179	14	max	.002	2	0	15	0	1	-8.916e-7	15	NC	1	NC	1
180		min	-.002	3	-.005	3	0	15	-2.402e-5	1	NC	1	NC	1
181	15	max	.001	2	0	15	0	1	-5.897e-7	15	NC	1	NC	1
182		min	-.002	3	-.004	3	0	15	-1.575e-5	1	NC	1	NC	1
183	16	max	.001	2	0	15	0	1	-1.692e-7	10	NC	1	NC	1
184		min	-.001	3	-.003	3	0	15	-7.485e-6	1	NC	1	NC	1
185	17	max	0	2	0	15	0	1	7.842e-7	1	NC	1	NC	1
186		min	0	3	-.002	3	0	15	-9.852e-7	3	NC	1	NC	1
187	18	max	0	2	0	15	0	1	9.054e-6	1	NC	1	NC	1
188		min	0	3	-.001	4	0	15	1.177e-7	12	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.732e-5	1	NC	1	NC	1
190		min	0	1	0	1	0	1	6.182e-7	15	NC	1	NC	1
191	M3	1	max	0	0	1	0	1	-1.937e-7	15	NC	1	NC	1
192		min	0	1	0	1	0	1	-5.398e-6	1	NC	1	NC	1
193	2	max	0	3	0	15	0	1	1.211e-5	1	NC	1	NC	1
194		min	0	2	-.002	4	0	15	4.407e-7	15	NC	1	NC	1
195	3	max	0	3	-.001	15	0	1	2.962e-5	1	NC	1	NC	1
196		min	0	2	-.005	4	0	15	1.075e-6	15	NC	1	NC	1
197	4	max	.001	3	-.002	15	0	1	4.713e-5	1	NC	1	NC	1
198		min	0	2	-.008	4	0	15	1.709e-6	15	NC	1	NC	1
199	5	max	.002	3	-.003	15	0	1	6.464e-5	1	NC	1	NC	1
200		min	-.001	2	-.011	4	0	15	2.344e-6	15	9259.558	4	NC	1
201	6	max	.002	3	-.003	15	0	1	8.215e-5	1	NC	1	NC	1
202		min	-.002	2	-.014	4	0	15	2.978e-6	15	7432.904	4	NC	1
203	7	max	.002	3	-.004	15	0	1	9.967e-5	1	NC	5	NC	1
204		min	-.002	2	-.016	4	0	15	3.612e-6	15	6336.815	4	NC	1
205	8	max	.003	3	-.004	15	0	1	1.172e-4	1	NC	5	NC	1
206		min	-.002	2	-.018	4	0	15	4.247e-6	15	5660.197	4	NC	1
207	9	max	.003	3	-.005	15	.001	1	1.347e-4	1	NC	5	NC	1
208		min	-.003	2	-.02	4	0	15	4.881e-6	15	5256.954	4	NC	1
209	10	max	.004	3	-.005	15	.001	1	1.522e-4	1	NC	5	NC	1
210		min	-.003	2	-.021	4	0	15	5.516e-6	15	5055.872	4	NC	1
211	11	max	.004	3	-.005	15	.002	1	1.697e-4	1	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.003	2	-.021	4	0	15	6.15e-6	15	5026.974	4	NC	1
213		max	.004	3	-.005	15	.002	1	1.872e-4	1	NC	5	NC	1
214		min	-.004	2	-.02	4	0	15	6.784e-6	15	5169.967	4	NC	1
215		max	.005	3	-.004	15	.002	1	2.047e-4	1	NC	5	NC	1
216		min	-.004	2	-.019	4	0	15	7.419e-6	15	5515.567	4	NC	1
217		max	.005	3	-.004	15	.003	1	2.222e-4	1	NC	5	NC	1
218		min	-.004	2	-.017	4	0	15	8.053e-6	15	6141.737	4	NC	1
219		max	.006	3	-.003	15	.003	1	2.397e-4	1	NC	2	NC	1
220		min	-.005	2	-.015	4	0	15	8.687e-6	15	7222.529	4	NC	1
221		max	.006	3	-.003	15	.004	1	2.573e-4	1	NC	1	NC	1
222		min	-.005	2	-.012	4	0	15	9.322e-6	15	9180.442	4	NC	1
223		max	.006	3	-.002	15	.004	1	2.748e-4	1	NC	1	NC	1
224		min	-.005	2	-.008	4	0	15	9.956e-6	15	NC	1	NC	1
225		max	.007	3	-.001	15	.005	1	2.923e-4	1	NC	1	NC	1
226		min	-.006	2	-.005	1	0	15	1.059e-5	15	NC	1	NC	1
227		max	.007	3	0	15	.006	1	3.098e-4	1	NC	1	NC	1
228		min	-.006	2	-.002	1	0	15	1.122e-5	15	NC	1	NC	1
229	M4	max	.003	1	.005	2	0	15	5.201e-5	1	NC	1	NC	2
230		min	0	3	-.007	3	-.006	1	1.906e-6	15	NC	1	4326.621	1
231		max	.003	1	.005	2	0	15	5.201e-5	1	NC	1	NC	2
232		min	0	3	-.007	3	-.005	1	1.906e-6	15	NC	1	4705.341	1
233		max	.002	1	.005	2	0	15	5.201e-5	1	NC	1	NC	2
234		min	0	3	-.007	3	-.005	1	1.906e-6	15	NC	1	5156.049	1
235		max	.002	1	.005	2	0	15	5.201e-5	1	NC	1	NC	2
236		min	0	3	-.006	3	-.004	1	1.906e-6	15	NC	1	5697.453	1
237		max	.002	1	.004	2	0	15	5.201e-5	1	NC	1	NC	2
238		min	0	3	-.006	3	-.004	1	1.906e-6	15	NC	1	6354.971	1
239		max	.002	1	.004	2	0	15	5.201e-5	1	NC	1	NC	2
240		min	0	3	-.005	3	-.003	1	1.906e-6	15	NC	1	7163.857	1
241		max	.002	1	.004	2	0	15	5.201e-5	1	NC	1	NC	2
242		min	0	3	-.005	3	-.003	1	1.906e-6	15	NC	1	8174.152	1
243		max	.002	1	.003	2	0	15	5.201e-5	1	NC	1	NC	2
244		min	0	3	-.004	3	-.003	1	1.906e-6	15	NC	1	9458.84	1
245		max	.001	1	.003	2	0	15	5.201e-5	1	NC	1	NC	1
246		min	0	3	-.004	3	-.002	1	1.906e-6	15	NC	1	NC	1
247		max	.001	1	.003	2	0	15	5.201e-5	1	NC	1	NC	1
248		min	0	3	-.004	3	-.002	1	1.906e-6	15	NC	1	NC	1
249		max	.001	1	.002	2	0	15	5.201e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.002	1	1.906e-6	15	NC	1	NC	1
251		max	.001	1	.002	2	0	15	5.201e-5	1	NC	1	NC	1
252		min	0	3	-.003	3	-.001	1	1.906e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	5.201e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	0	1	1.906e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	5.201e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	1.906e-6	15	NC	1	NC	1
257		max	0	1	.001	2	0	15	5.201e-5	1	NC	1	NC	1
258		min	0	3	-.002	3	0	1	1.906e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	5.201e-5	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	1.906e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	5.201e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	1.906e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	5.201e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	1.906e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	5.201e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	1.906e-6	15	NC	1	NC	1
267	M6	max	.019	2	.027	2	0	1	0	1	NC	3	NC	1
268		min	-.026	3	-.038	3	0	1	0	1	2258.288	2	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.018	2	.025	2	0	1	0	1	NC	3	NC	1
270		min	-.024	3	-.036	3	0	1	0	1	2468.427	2	NC	1
271	3	max	.017	2	.022	2	0	1	0	1	NC	3	NC	1
272		min	-.023	3	-.034	3	0	1	0	1	2719.867	2	NC	1
273	4	max	.016	2	.02	2	0	1	0	1	NC	3	NC	1
274		min	-.021	3	-.032	3	0	1	0	1	3023.845	2	NC	1
275	5	max	.015	2	.018	2	0	1	0	1	NC	3	NC	1
276		min	-.02	3	-.03	3	0	1	0	1	3395.817	2	NC	1
277	6	max	.014	2	.016	2	0	1	0	1	NC	3	NC	1
278		min	-.019	3	-.028	3	0	1	0	1	3857.512	2	NC	1
279	7	max	.013	2	.014	2	0	1	0	1	NC	3	NC	1
280		min	-.017	3	-.025	3	0	1	0	1	4440.267	2	NC	1
281	8	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
282		min	-.016	3	-.023	3	0	1	0	1	5190.646	2	NC	1
283	9	max	.011	2	.01	2	0	1	0	1	NC	1	NC	1
284		min	-.014	3	-.021	3	0	1	0	1	6180.312	2	NC	1
285	10	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
286		min	-.013	3	-.019	3	0	1	0	1	7524.299	2	NC	1
287	11	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
288		min	-.011	3	-.017	3	0	1	0	1	9417.007	2	NC	1
289	12	max	.007	2	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.01	3	-.015	3	0	1	0	1	NC	1	NC	1
291	13	max	.006	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.009	3	-.013	3	0	1	0	1	NC	1	NC	1
293	14	max	.005	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.007	3	-.011	3	0	1	0	1	NC	1	NC	1
295	15	max	.004	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.006	3	-.008	3	0	1	0	1	NC	1	NC	1
297	16	max	.003	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.006	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.004	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.004	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	-.001	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.007	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.01	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.003	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.013	3	0	1	0	1	8276.466	3	NC	1
315	6	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.016	3	0	1	0	1	6967.833	3	NC	1
317	7	max	.007	3	-.004	15	0	1	0	1	NC	1	NC	1
318		min	-.007	2	-.018	3	0	1	0	1	6180.997	3	NC	1
319	8	max	.008	3	-.004	15	0	1	0	1	NC	2	NC	1
320		min	-.008	2	-.019	3	0	1	0	1	5709.011	3	NC	1
321	9	max	.01	3	-.005	15	0	1	0	1	NC	2	NC	1
322		min	-.009	2	-.02	3	0	1	0	1	5340.642	4	NC	1
323	10	max	.011	3	-.005	15	0	1	0	1	NC	2	NC	1
324		min	-.01	2	-.021	3	0	1	0	1	5131.375	4	NC	1
325	11	max	.012	3	-.005	15	0	1	0	1	NC	5	NC	1



Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.011	2	-.021	4	0	1	0	1	5097.907	4	NC	1
327		12	max	.013	3	-.005	15	0	1	0	1	NC	5	NC	1
328			min	-.012	2	-.02	4	0	1	0	1	5239.355	4	NC	1
329		13	max	.014	3	-.004	15	0	1	0	1	NC	2	NC	1
330			min	-.013	2	-.019	4	0	1	0	1	5586.419	4	NC	1
331		14	max	.016	3	-.004	15	0	1	0	1	NC	2	NC	1
332			min	-.014	2	-.017	4	0	1	0	1	6217.708	4	NC	1
333		15	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
334			min	-.015	2	-.015	4	0	1	0	1	7309.075	4	NC	1
335		16	max	.018	3	-.003	15	0	1	0	1	NC	1	NC	1
336			min	-.016	2	-.012	3	0	1	0	1	9287.664	4	NC	1
337		17	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
338			min	-.017	2	-.009	3	0	1	0	1	NC	1	NC	1
339		18	max	.02	3	-.001	15	0	1	0	1	NC	1	NC	1
340			min	-.018	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.021	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.02	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	2	.018	2	0	1	0	1	NC	1	NC	1
344			min	-.002	3	-.022	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	2	.017	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.02	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	2	.016	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.019	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	2	.015	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.018	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	2	.014	2	0	1	0	1	NC	1	NC	1
352			min	-.002	3	-.017	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	2	.013	2	0	1	0	1	NC	1	NC	1
354			min	-.002	3	-.016	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	2	.012	2	0	1	0	1	NC	1	NC	1
356			min	-.001	3	-.014	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	2	.011	2	0	1	0	1	NC	1	NC	1
358			min	-.001	3	-.013	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	2	.01	2	0	1	0	1	NC	1	NC	1
360			min	-.001	3	-.012	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	2	.009	2	0	1	0	1	NC	1	NC	1
362			min	-.001	3	-.011	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	2	.008	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	2	.007	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	2	.006	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.005	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	2	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	2	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	2	.008	2	0	15	1.315e-4	1	NC	1	NC	2
382			min	-.008	3	-.013	3	-.006	1	4.817e-6	15	7464.909	2	9971.616	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.006	2	.007	2	0	15	1.233e-4	1	NC	1	NC	1
384			min	-.008	3	-.012	3	-.006	1	4.515e-6	15	8563.978	2	NC	1
385		3	max	.006	2	.006	2	0	15	1.15e-4	1	NC	1	NC	1
386			min	-.007	3	-.012	3	-.005	1	4.213e-6	15	NC	1	NC	1
387		4	max	.005	2	.005	2	0	15	1.067e-4	1	NC	1	NC	1
388			min	-.007	3	-.011	3	-.005	1	3.911e-6	15	NC	1	NC	1
389		5	max	.005	2	.004	2	0	15	9.845e-5	1	NC	1	NC	1
390			min	-.006	3	-.011	3	-.004	1	3.609e-6	15	NC	1	NC	1
391		6	max	.005	2	.003	2	0	15	9.018e-5	1	NC	1	NC	1
392			min	-.006	3	-.01	3	-.004	1	3.307e-6	15	NC	1	NC	1
393		7	max	.004	2	.002	2	0	15	8.191e-5	1	NC	1	NC	1
394			min	-.006	3	-.01	3	-.003	1	3.005e-6	15	NC	1	NC	1
395		8	max	.004	2	.001	2	0	15	7.364e-5	1	NC	1	NC	1
396			min	-.005	3	-.009	3	-.003	1	2.703e-6	15	NC	1	NC	1
397		9	max	.004	2	0	2	0	15	6.537e-5	1	NC	1	NC	1
398			min	-.005	3	-.009	3	-.002	1	2.401e-6	15	NC	1	NC	1
399		10	max	.003	2	0	2	0	15	5.71e-5	1	NC	1	NC	1
400			min	-.004	3	-.008	3	-.002	1	2.099e-6	15	NC	1	NC	1
401		11	max	.003	2	0	2	0	15	4.883e-5	1	NC	1	NC	1
402			min	-.004	3	-.007	3	-.002	1	1.797e-6	15	NC	1	NC	1
403		12	max	.002	2	0	2	0	15	4.056e-5	1	NC	1	NC	1
404			min	-.003	3	-.007	3	-.001	1	1.496e-6	15	NC	1	NC	1
405		13	max	.002	2	-.001	15	0	15	3.229e-5	1	NC	1	NC	1
406			min	-.003	3	-.006	3	0	1	1.194e-6	15	NC	1	NC	1
407		14	max	.002	2	0	15	0	15	2.402e-5	1	NC	1	NC	1
408			min	-.002	3	-.005	3	0	1	8.916e-7	15	NC	1	NC	1
409		15	max	.001	2	0	15	0	15	1.575e-5	1	NC	1	NC	1
410			min	-.002	3	-.004	3	0	1	5.897e-7	15	NC	1	NC	1
411		16	max	.001	2	0	15	0	15	7.485e-6	1	NC	1	NC	1
412			min	-.001	3	-.003	3	0	1	1.692e-7	10	NC	1	NC	1
413		17	max	0	2	0	15	0	15	9.852e-7	3	NC	1	NC	1
414			min	0	3	-.002	3	0	1	-7.842e-7	1	NC	1	NC	1
415		18	max	0	2	0	15	0	15	-1.177e-7	12	NC	1	NC	1
416			min	0	3	-.001	4	0	1	-9.054e-6	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	-6.182e-7	15	NC	1	NC	1
418			min	0	1	0	1	0	1	-1.732e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	5.398e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	1.937e-7	15	NC	1	NC	1
421		2	max	0	3	0	15	0	15	-4.407e-7	15	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.211e-5	1	NC	1	NC	1
423		3	max	0	3	-.001	15	0	15	-1.075e-6	15	NC	1	NC	1
424			min	0	2	-.005	4	0	1	-2.962e-5	1	NC	1	NC	1
425		4	max	.001	3	-.002	15	0	15	-1.709e-6	15	NC	1	NC	1
426			min	0	2	-.008	4	0	1	-4.713e-5	1	NC	1	NC	1
427		5	max	.002	3	-.003	15	0	15	-2.344e-6	15	NC	1	NC	1
428			min	-.001	2	-.011	4	0	1	-6.464e-5	1	9259.558	4	NC	1
429		6	max	.002	3	-.003	15	0	15	-2.978e-6	15	NC	1	NC	1
430			min	-.002	2	-.014	4	0	1	-8.215e-5	1	7432.904	4	NC	1
431		7	max	.002	3	-.004	15	0	15	-3.612e-6	15	NC	5	NC	1
432			min	-.002	2	-.016	4	0	1	-9.967e-5	1	6336.815	4	NC	1
433		8	max	.003	3	-.004	15	0	15	-4.247e-6	15	NC	5	NC	1
434			min	-.002	2	-.018	4	0	1	-1.172e-4	1	5660.197	4	NC	1
435		9	max	.003	3	-.005	15	0	15	-4.881e-6	15	NC	5	NC	1
436			min	-.003	2	-.02	4	-.001	1	-1.347e-4	1	5256.954	4	NC	1
437		10	max	.004	3	-.005	15	0	15	-5.516e-6	15	NC	5	NC	1
438			min	-.003	2	-.021	4	-.001	1	-1.522e-4	1	5055.872	4	NC	1
439		11	max	.004	3	-.005	15	0	15	-6.15e-6	15	NC	5	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.003	2	-.021	4	-.002	1	-1.697e-4	1	5026.974	4	NC	1
441		max	.004	3	-.005	15	0	15	-6.784e-6	15	NC	5	NC	1
442		min	-.004	2	-.02	4	-.002	1	-1.872e-4	1	5169.967	4	NC	1
443		max	.005	3	-.004	15	0	15	-7.419e-6	15	NC	5	NC	1
444		min	-.004	2	-.019	4	-.002	1	-2.047e-4	1	5515.567	4	NC	1
445		max	.005	3	-.004	15	0	15	-8.053e-6	15	NC	5	NC	1
446		min	-.004	2	-.017	4	-.003	1	-2.222e-4	1	6141.737	4	NC	1
447		max	.006	3	-.003	15	0	15	-8.687e-6	15	NC	2	NC	1
448		min	-.005	2	-.015	4	-.003	1	-2.397e-4	1	7222.529	4	NC	1
449		max	.006	3	-.003	15	0	15	-9.322e-6	15	NC	1	NC	1
450		min	-.005	2	-.012	4	-.004	1	-2.573e-4	1	9180.442	4	NC	1
451		max	.006	3	-.002	15	0	15	-9.956e-6	15	NC	1	NC	1
452		min	-.005	2	-.008	4	-.004	1	-2.748e-4	1	NC	1	NC	1
453		max	.007	3	-.001	15	0	15	-1.059e-5	15	NC	1	NC	1
454		min	-.006	2	-.005	1	-.005	1	-2.923e-4	1	NC	1	NC	1
455		max	.007	3	0	15	0	15	-1.122e-5	15	NC	1	NC	1
456		min	-.006	2	-.002	1	-.006	1	-3.098e-4	1	NC	1	NC	1
457	M12	max	.003	1	.005	2	.006	1	-1.906e-6	15	NC	1	NC	2
458		min	0	3	-.007	3	0	15	-5.201e-5	1	NC	1	4326.621	1
459		max	.003	1	.005	2	.005	1	-1.906e-6	15	NC	1	NC	2
460		min	0	3	-.007	3	0	15	-5.201e-5	1	NC	1	4705.341	1
461		max	.002	1	.005	2	.005	1	-1.906e-6	15	NC	1	NC	2
462		min	0	3	-.007	3	0	15	-5.201e-5	1	NC	1	5156.049	1
463		max	.002	1	.005	2	.004	1	-1.906e-6	15	NC	1	NC	2
464		min	0	3	-.006	3	0	15	-5.201e-5	1	NC	1	5697.453	1
465		max	.002	1	.004	2	.004	1	-1.906e-6	15	NC	1	NC	2
466		min	0	3	-.006	3	0	15	-5.201e-5	1	NC	1	6354.971	1
467		max	.002	1	.004	2	.003	1	-1.906e-6	15	NC	1	NC	2
468		min	0	3	-.005	3	0	15	-5.201e-5	1	NC	1	7163.857	1
469		max	.002	1	.004	2	.003	1	-1.906e-6	15	NC	1	NC	2
470		min	0	3	-.005	3	0	15	-5.201e-5	1	NC	1	8174.152	1
471		max	.002	1	.003	2	.003	1	-1.906e-6	15	NC	1	NC	2
472		min	0	3	-.004	3	0	15	-5.201e-5	1	NC	1	9458.84	1
473		max	.001	1	.003	2	.002	1	-1.906e-6	15	NC	1	NC	1
474		min	0	3	-.004	3	0	15	-5.201e-5	1	NC	1	NC	1
475		max	.001	1	.003	2	.002	1	-1.906e-6	15	NC	1	NC	1
476		min	0	3	-.004	3	0	15	-5.201e-5	1	NC	1	NC	1
477		max	.001	1	.002	2	.002	1	-1.906e-6	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-5.201e-5	1	NC	1	NC	1
479		max	.001	1	.002	2	.001	1	-1.906e-6	15	NC	1	NC	1
480		min	0	3	-.003	3	0	15	-5.201e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-1.906e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-5.201e-5	1	NC	1	NC	1
483		max	0	1	.002	2	0	1	-1.906e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-5.201e-5	1	NC	1	NC	1
485		max	0	1	.001	2	0	1	-1.906e-6	15	NC	1	NC	1
486		min	0	3	-.002	3	0	15	-5.201e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-1.906e-6	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-5.201e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-1.906e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-5.201e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-1.906e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-5.201e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-1.906e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-5.201e-5	1	NC	1	NC	1
495	M1	max	.009	3	.229	2	0	1	6.97e-3	1	NC	1	NC	1
496		min	-.006	2	-.067	3	0	15	-1.561e-2	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.009	3	.112	2	0	15	3.361e-3	1	NC	5	NC	1
498			min	-.006	2	-.034	3	-.004	1	-7.748e-3	3	1167.024	2	NC	1
499		3	max	.009	3	.013	3	0	15	2.6e-5	10	NC	5	NC	1
500			min	-.006	2	-.011	2	-.006	1	-1.159e-4	1	565.737	2	NC	1
501		4	max	.009	3	.083	3	0	15	3.97e-3	2	NC	15	NC	1
502			min	-.005	2	-.147	2	-.006	1	-3.883e-3	3	360.568	2	NC	1
503		5	max	.009	3	.17	3	0	15	7.934e-3	2	NC	15	NC	1
504			min	-.005	2	-.288	2	-.004	1	-7.673e-3	3	262.251	2	NC	1
505		6	max	.009	3	.261	3	0	15	1.19e-2	2	8339.099	15	NC	1
506			min	-.005	2	-.423	2	-.002	1	-1.146e-2	3	207.774	2	NC	1
507		7	max	.009	3	.348	3	0	1	1.586e-2	2	7057.992	15	NC	1
508			min	-.005	2	-.543	2	0	3	-1.525e-2	3	175.47	2	NC	1
509		8	max	.008	3	.42	3	0	1	1.983e-2	2	6298.1	15	NC	1
510			min	-.005	2	-.639	2	0	15	-1.904e-2	3	156.304	2	NC	1
511		9	max	.008	3	.467	3	0	15	2.212e-2	2	5899.6	15	NC	1
512			min	-.005	2	-.699	2	0	1	-1.962e-2	3	146.294	2	NC	1
513		10	max	.008	3	.485	3	0	1	2.33e-2	2	5777.543	15	NC	1
514			min	-.005	2	-.719	2	0	15	-1.805e-2	3	143.357	2	NC	1
515		11	max	.008	3	.474	3	0	1	2.448e-2	2	5899.285	15	NC	1
516			min	-.005	2	-.698	2	0	15	-1.648e-2	3	146.787	2	NC	1
517		12	max	.008	3	.434	3	0	15	2.333e-2	2	6297.419	15	NC	1
518			min	-.005	2	-.636	2	0	1	-1.438e-2	3	157.74	2	NC	1
519		13	max	.007	3	.37	3	0	10	1.871e-2	2	7056.78	15	NC	1
520			min	-.005	2	-.537	2	0	1	-1.151e-2	3	178.836	2	NC	1
521		14	max	.007	3	.288	3	.001	1	1.408e-2	2	8337.033	15	NC	1
522			min	-.004	2	-.413	2	0	15	-8.635e-3	3	214.772	2	NC	1
523		15	max	.007	3	.196	3	.004	1	9.454e-3	2	NC	15	NC	1
524			min	-.004	2	-.276	2	0	15	-5.762e-3	3	276.298	2	NC	1
525		16	max	.007	3	.099	3	.005	1	4.828e-3	2	NC	15	NC	1
526			min	-.004	2	-.136	2	0	15	-2.889e-3	3	389.387	2	NC	1
527		17	max	.007	3	.005	3	.006	1	4.029e-4	1	NC	5	NC	1
528			min	-.004	2	-.006	2	0	15	-1.534e-5	3	629.043	2	NC	1
529		18	max	.007	3	.104	2	.004	1	5.544e-3	2	NC	5	NC	1
530			min	-.004	2	-.081	3	0	15	-1.84e-3	3	1325.528	2	NC	1
531		19	max	.007	3	.204	2	0	15	1.107e-2	2	NC	1	NC	1
532			min	-.004	2	-.162	3	0	1	-3.748e-3	3	NC	1	NC	1
533	M5	1	max	.028	3	.365	2	0	1	0	1	NC	1	NC	1
534			min	-.019	2	-.016	3	0	1	0	1	NC	1	NC	1
535		2	max	.028	3	.18	2	0	1	0	1	NC	5	NC	1
536			min	-.019	2	-.01	3	0	1	0	1	744.014	2	NC	1
537		3	max	.028	3	.038	3	0	1	0	1	NC	5	NC	1
538			min	-.019	2	-.031	2	0	1	0	1	345.863	2	NC	1
539		4	max	.027	3	.164	3	0	1	0	1	9403.62	15	NC	1
540			min	-.019	2	-.29	2	0	1	0	1	208.643	2	NC	1
541		5	max	.026	3	.346	3	0	1	0	1	6518.263	15	NC	1
542			min	-.018	2	-.576	2	0	1	0	1	145.052	2	NC	1
543		6	max	.026	3	.554	3	0	1	0	1	4983.761	15	NC	1
544			min	-.018	2	-.862	2	0	1	0	1	111.091	2	NC	1
545		7	max	.025	3	.759	3	0	1	0	1	4103.948	15	NC	1
546			min	-.018	2	-1.124	2	0	1	0	1	91.554	2	NC	1
547		8	max	.025	3	.932	3	0	1	0	1	3595.271	15	NC	1
548			min	-.017	2	-1.334	2	0	1	0	1	80.232	2	NC	1
549		9	max	.024	3	1.044	3	0	1	0	1	3335.165	15	NC	1
550			min	-.017	2	-1.468	2	0	1	0	1	74.434	2	NC	1
551		10	max	.024	3	1.085	3	0	1	0	1	3256.848	15	NC	1
552			min	-.017	2	-1.514	2	0	1	0	1	72.74	2	NC	1
553		11	max	.023	3	1.058	3	0	1	0	1	3335.366	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.016	2	-1.47	2	0	1	0	1	74.714	2	NC	1
555		12	max	.022	3	.965	3	0	1	0	1	3595.738	15	NC	1
556			min	-.016	2	-1.331	2	0	1	0	1	81.166	2	NC	1
557		13	max	.022	3	.814	3	0	1	0	1	4104.866	15	NC	1
558			min	-.016	2	-1.109	2	0	1	0	1	94.022	2	NC	1
559		14	max	.021	3	.625	3	0	1	0	1	4985.506	15	NC	1
560			min	-.015	2	-.835	2	0	1	0	1	116.775	2	NC	1
561		15	max	.021	3	.416	3	0	1	0	1	6521.654	15	NC	1
562			min	-.015	2	-.541	2	0	1	0	1	157.774	2	NC	1
563		16	max	.02	3	.206	3	0	1	0	1	9410.672	15	NC	1
564			min	-.015	2	-.258	2	0	1	0	1	238.4	2	NC	1
565		17	max	.02	3	.013	3	0	1	0	1	NC	5	NC	1
566			min	-.015	2	-.017	2	0	1	0	1	422.282	2	NC	1
567		18	max	.02	3	.159	1	0	1	0	1	NC	5	NC	1
568			min	-.015	2	-.148	3	0	1	0	1	945.402	1	NC	1
569		19	max	.02	3	.299	1	0	1	0	1	NC	1	NC	1
570			min	-.015	2	-.29	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.009	3	.229	2	0	15	1.561e-2	3	NC	1	NC	1
572			min	-.006	2	-.067	3	0	1	-6.97e-3	1	NC	1	NC	1
573		2	max	.009	3	.112	2	.004	1	7.748e-3	3	NC	5	NC	1
574			min	-.006	2	-.034	3	0	15	-3.361e-3	1	1167.024	2	NC	1
575		3	max	.009	3	.013	3	.006	1	1.159e-4	1	NC	5	NC	1
576			min	-.006	2	-.011	2	0	15	-2.6e-5	10	565.737	2	NC	1
577		4	max	.009	3	.083	3	.006	1	3.883e-3	3	NC	15	NC	1
578			min	-.005	2	-.147	2	0	15	-3.97e-3	2	360.568	2	NC	1
579		5	max	.009	3	.17	3	.004	1	7.673e-3	3	NC	15	NC	1
580			min	-.005	2	-.288	2	0	15	-7.934e-3	2	262.251	2	NC	1
581		6	max	.009	3	.261	3	.002	1	1.146e-2	3	8339.099	15	NC	1
582			min	-.005	2	-.423	2	0	15	-1.19e-2	2	207.774	2	NC	1
583		7	max	.009	3	.348	3	0	3	1.525e-2	3	7057.992	15	NC	1
584			min	-.005	2	-.543	2	0	1	-1.586e-2	2	175.47	2	NC	1
585		8	max	.008	3	.42	3	0	15	1.904e-2	3	6298.1	15	NC	1
586			min	-.005	2	-.639	2	0	1	-1.983e-2	2	156.304	2	NC	1
587		9	max	.008	3	.467	3	0	1	1.962e-2	3	5899.6	15	NC	1
588			min	-.005	2	-.699	2	0	15	-2.212e-2	2	146.294	2	NC	1
589		10	max	.008	3	.485	3	0	15	1.805e-2	3	5777.543	15	NC	1
590			min	-.005	2	-.719	2	0	1	-2.33e-2	2	143.357	2	NC	1
591		11	max	.008	3	.474	3	0	15	1.648e-2	3	5899.285	15	NC	1
592			min	-.005	2	-.698	2	0	1	-2.448e-2	2	146.787	2	NC	1
593		12	max	.008	3	.434	3	0	1	1.438e-2	3	6297.419	15	NC	1
594			min	-.005	2	-.636	2	0	15	-2.333e-2	2	157.74	2	NC	1
595		13	max	.007	3	.37	3	0	1	1.151e-2	3	7056.78	15	NC	1
596			min	-.005	2	-.537	2	0	10	-1.871e-2	2	178.836	2	NC	1
597		14	max	.007	3	.288	3	0	15	8.635e-3	3	8337.033	15	NC	1
598			min	-.004	2	-.413	2	-.001	1	-1.408e-2	2	214.772	2	NC	1
599		15	max	.007	3	.196	3	0	15	5.762e-3	3	NC	15	NC	1
600			min	-.004	2	-.276	2	-.004	1	-9.454e-3	2	276.298	2	NC	1
601		16	max	.007	3	.099	3	0	15	2.889e-3	3	NC	15	NC	1
602			min	-.004	2	-.136	2	-.005	1	-4.828e-3	2	389.387	2	NC	1
603		17	max	.007	3	.005	3	0	15	1.534e-5	3	NC	5	NC	1
604			min	-.004	2	-.006	2	-.006	1	-4.029e-4	1	629.043	2	NC	1
605		18	max	.007	3	.104	2	0	15	1.84e-3	3	NC	5	NC	1
606			min	-.004	2	-.081	3	-.004	1	-5.544e-3	2	1325.528	2	NC	1
607		19	max	.007	3	.204	2	0	1	3.748e-3	3	NC	1	NC	1
608			min	-.004	2	-.162	3	0	15	-1.107e-2	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

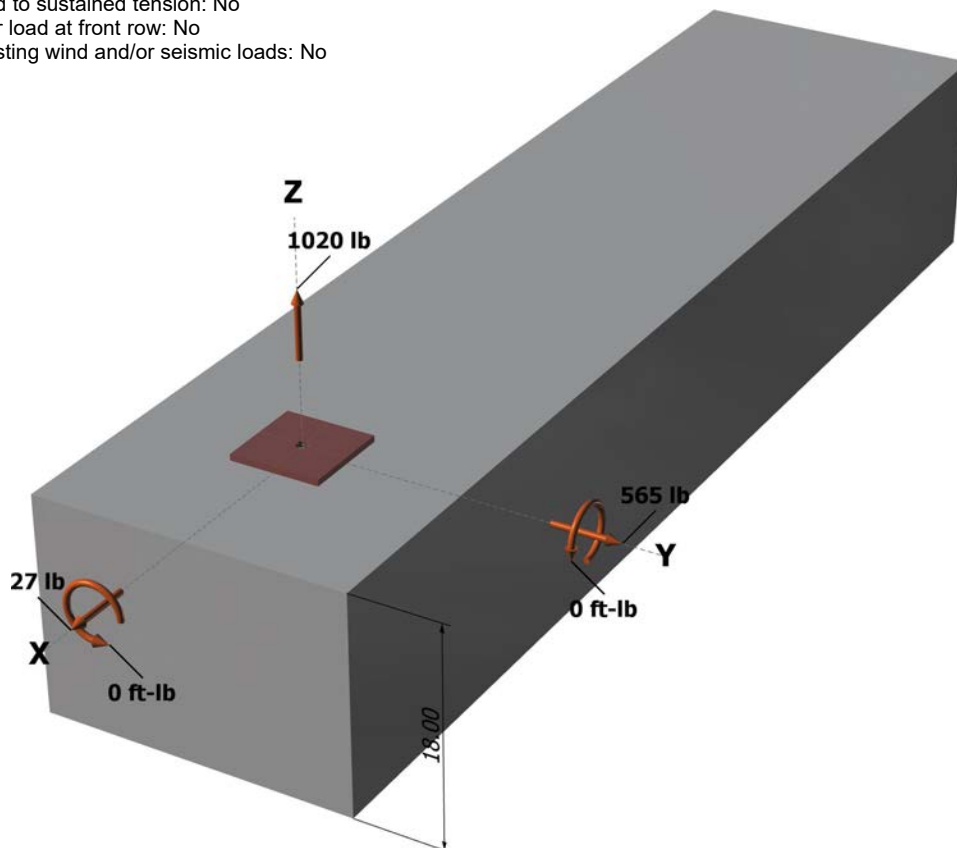
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (‰): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1020
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbv} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbv} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbv} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbv} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
Adhesive	1020	5365	0.19	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	566	3156	0.18	Pass (Governs)	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 c_{ac} (inch): 9.67
 c_{min} (inch): 1.75
 s_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

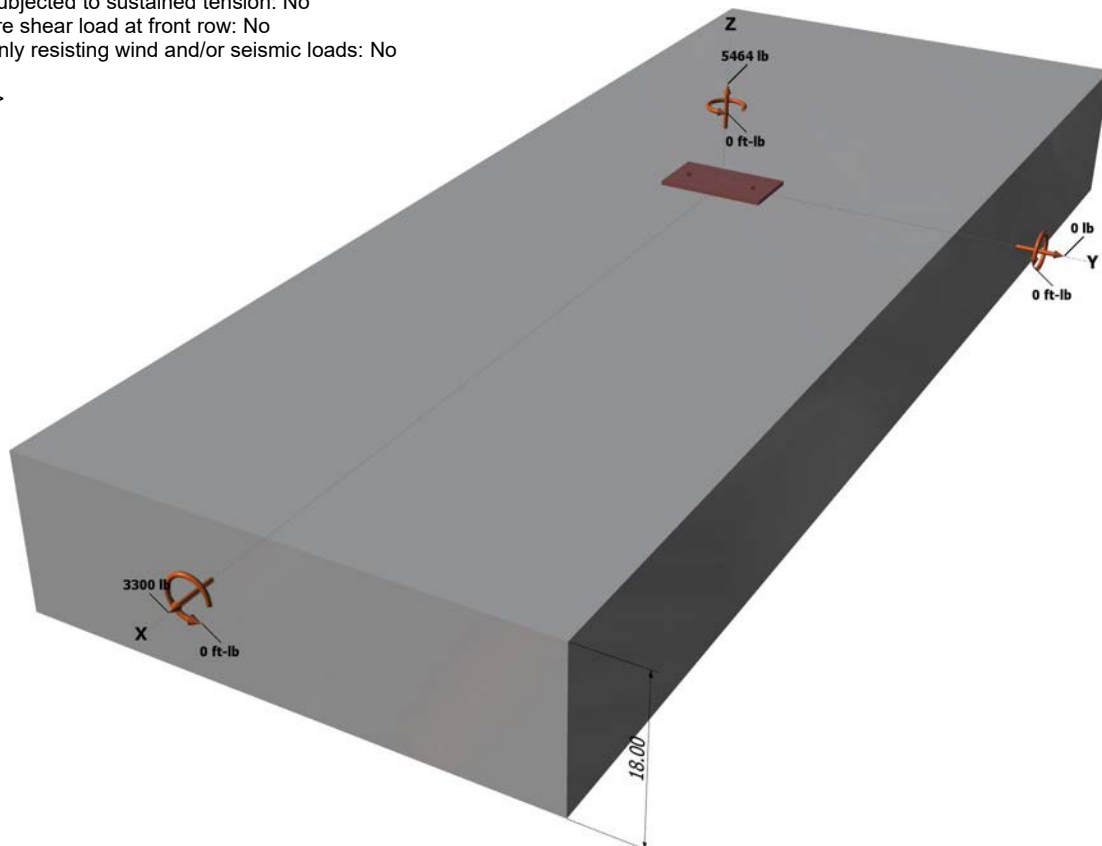
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

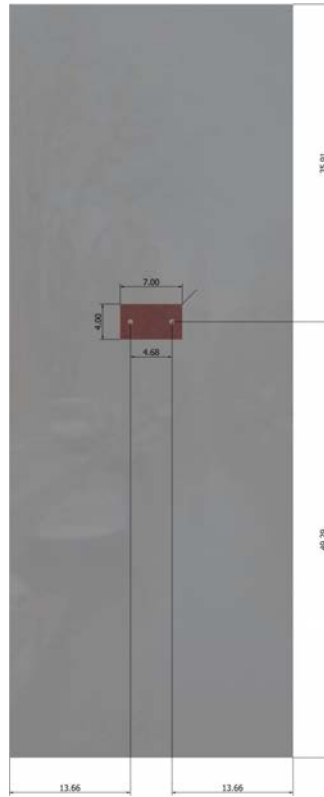
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2732.0	1650.0	0.0	1650.0
2	2732.0	1650.0	0.0	1650.0
Sum	5464.0	3300.0	0.0	3300.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 5464

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{ short-term } K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
576.00	648.00	1.000	0.928	1.000	1.000	15593	0.70	9001

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}^{1.5}} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.66	18939

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
737.64	839.68	1.000	1.000	1.000	18939	0.70	23292

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cp} = 20601$$

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	2732	6071	0.45	Pass
Concrete breakout	5464	10231	0.53	Pass
Adhesive	5464	8093	0.68	Pass (Governs)
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	1650	3156	0.52	Pass (Governs)
T Concrete breakout x+	3300	9001	0.37	Pass

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 32-40 Inch Width		
Address:			
Phone:			
E-mail:			

Concrete breakout y-	1650	23292	0.07	Pass
Pryout	3300	20601	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.68	0.52	119.8 %	1.2	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.