

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	25° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2
Module Tilt = 25°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf

Self-weight of the PV modules.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	18.56 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.82	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	100 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 15.70 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.100	(Pressure)
$C_{f+ BOTTOM}$ =	1.700	
$C_{f- TOP, OUTER PURLIN}$ =	-2.500	
$C_{f- TOP, INNER PURLIN}$ =	-1.900	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	S1.5
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	123 in
ΦF_{ty} STRONG-AXIS =	25.07 ksi
ΦF_{ty} WEAK-AXIS =	23.08 ksi
S_y =	1.33 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	2.16 in ⁴
I_x =	1.07 in ⁴
A =	1.25 in ²
g =	1.50 lbs/ft
M_y =	1.861 k-ft
M_z =	0.346 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	97%



DETAIL VIEW

4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	BF0
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	88.90 in
ΦF_{ty} AXIAL =	31.09 ksi
ΦF_{ty} STRONG-AXIS =	29.35 ksi
ΦF_{ty} WEAK-AXIS =	33.25 ksi
S_y =	1.42 in ³
S_x =	1.41 in ³
E =	10100 ksi
I_y =	2.39 in ⁴
I_x =	2.22 in ⁴
A =	1.88 in ²
g =	2.26 lbs/ft
M_y =	-2.897 k-ft
M_z =	0.000 k-ft
P_n =	-0.685 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	85%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	3.057 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	11%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	0.010 k-ft
M_z =	0.000 k-ft
P_n =	1.981 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	28%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	55x55
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	63.42 in
$\Phi F_{ty \text{ AXIAL}}$ =	12.77 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
S_y =	0.60 in ³
S_x =	0.60 in ³
E =	10100 ksi
I_y =	0.67 in ⁴
I_x =	0.67 in ⁴
A =	0.98 in ²
g =	1.18 lbs/ft
M_y =	-0.009 k-ft
M_z =	0.000 k-ft
P_n =	3.243 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	12.545 k
Utilization =	26%



5. FOUNDATION DESIGN CALCULATIONS

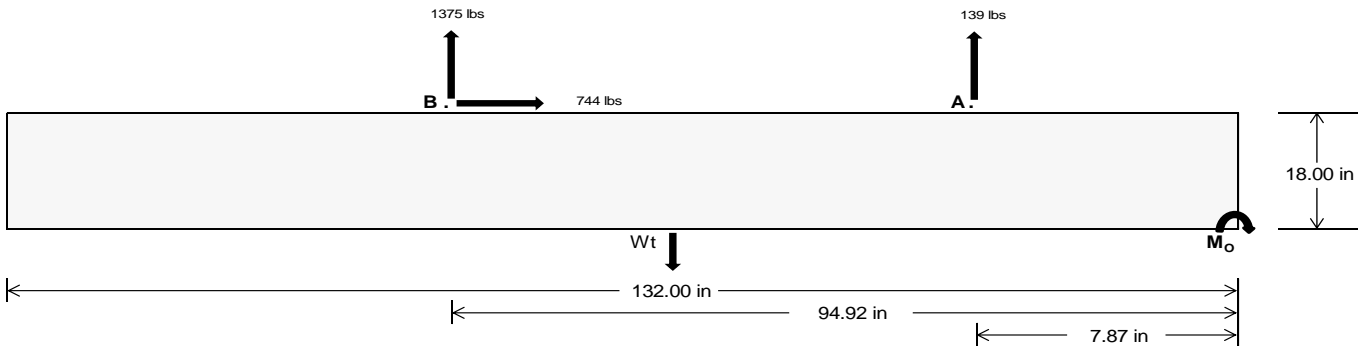
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>590.87</u>	<u>5733.53</u> k
Compressive Load =		<u>3974.34</u>	<u>4706.00</u> k
Lateral Load =		<u>14.58</u>	<u>3096.70</u> k
Moment (Weak Axis) =		<u>0.03</u>	<u>0.01</u> k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 145037.7$ in-lbs
Resisting Force Required = 2197.54 lbs
S.F. = 1.67
Weight Required = 3662.57 lbs
Minimum Width = 31 in
Weight Provided = 6180.63 lbs

Sliding

Force = 744.13 lbs
Friction = 0.4
Weight Required = 1860.32 lbs
Resisting Weight = 6180.63 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 744.13 lbs
Cohesion = 130 psf
Area = 28.42 ft²
Resisting = 3090.31 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 31in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 31in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 31in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
31 in 32 in 33 in 34 in
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.58 \text{ ft}) =$ 6181 lbs 6380 lbs 6579 lbs 6779 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in	31 in	32 in	33 in	34 in
F_A	1421 lbs	1421 lbs	1421 lbs	1421 lbs	1379 lbs	1379 lbs	1379 lbs	1379 lbs	1967 lbs	1967 lbs	1967 lbs	1967 lbs	-278 lbs	-278 lbs	-278 lbs	-278 lbs
F_B	1411 lbs	1411 lbs	1411 lbs	1411 lbs	1968 lbs	1968 lbs	1968 lbs	1968 lbs	2404 lbs	2404 lbs	2404 lbs	2404 lbs	-2751 lbs	-2751 lbs	-2751 lbs	-2751 lbs
F_V	193 lbs	193 lbs	193 lbs	193 lbs	1346 lbs	1346 lbs	1346 lbs	1346 lbs	1137 lbs	1137 lbs	1137 lbs	1137 lbs	-1488 lbs	-1488 lbs	-1488 lbs	-1488 lbs
P_{total}	9012 lbs	9212 lbs	9411 lbs	9610 lbs	9527 lbs	9727 lbs	9926 lbs	10125 lbs	10552 lbs	10751 lbs	10950 lbs	11150 lbs	680 lbs	800 lbs	919 lbs	1039 lbs
M	3769 lbs-ft	3769 lbs-ft	3769 lbs-ft	3769 lbs-ft	3953 lbs-ft	3953 lbs-ft	3953 lbs-ft	3953 lbs-ft	5442 lbs-ft	5442 lbs-ft	5442 lbs-ft	5442 lbs-ft	3052 lbs-ft	3052 lbs-ft	3052 lbs-ft	3052 lbs-ft
e	0.42 ft	0.41 ft	0.40 ft	0.39 ft	0.41 ft	0.41 ft	0.40 ft	0.39 ft	0.52 ft	0.51 ft	0.50 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	244.8 psf	243.9 psf	243.1 psf	242.4 psf	259.4 psf	258.1 psf	256.8 psf	255.7 psf	266.9 psf	265.3 psf	263.9 psf	262.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	389.5 psf	384.1 psf	379.1 psf	374.3 psf	411.2 psf	405.1 psf	399.4 psf	394.1 psf	475.8 psf	467.7 psf	460.1 psf	453.0 psf	173.5 psf	118.8 psf	102.2 psf	95.4 psf

Maximum Bearing Pressure = 476 psf
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 31in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

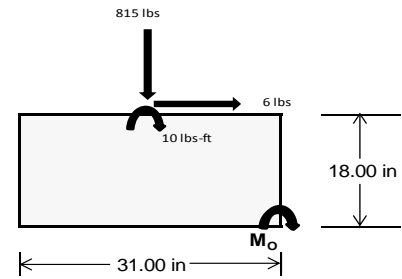
Overturning Check

$M_o = 1034.4 \text{ ft-lbs}$
 Resisting Force Required = 800.83 lbs
 S.F. = 1.67
 Weight Required = 1334.72 lbs
 Minimum Width = **31 in**
 Weight Provided = 6180.63 lbs

A minimum 132in long x 31in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	31 in			31 in			31 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	250 lbs	650 lbs	250 lbs	815 lbs	2349 lbs	815 lbs	73 lbs	190 lbs	73 lbs
F_v	2 lbs	0 lbs	2 lbs	6 lbs	0 lbs	6 lbs	0 lbs	0 lbs	0 lbs
P_{total}	7901 lbs	6181 lbs	7901 lbs	8099 lbs	6181 lbs	8099 lbs	2310 lbs	6181 lbs	2310 lbs
M	5 lbs-ft	0 lbs-ft	5 lbs-ft	19 lbs-ft	0 lbs-ft	19 lbs-ft	1 lbs-ft	0 lbs-ft	1 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft	0.43 ft
f_{min}	277.6 psf	217.5 psf	277.6 psf	283.5 psf	217.5 psf	283.5 psf	81.3 psf	217.5 psf	81.3 psf
f_{max}	278.5 psf	217.5 psf	278.5 psf	286.5 psf	217.5 psf	286.5 psf	81.4 psf	217.5 psf	81.4 psf



Maximum Bearing Pressure = 287 psf
 Allowable Bearing Pressure = 1500 psf

Use a 132in long x 31in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 132in long x 31in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.588 k
Allowable Uplift =	1.214 k
Utilization =	<u>48%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	2.142 k
Allowable Uplift =	4.357 k
Utilization =	<u>49%</u>



6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	3.057 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>41%</u>

Rear Strut

Maximum Axial Load =	3.876 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>52%</u>

Diagonal Strut

Maximum Axial Load =	2.062 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>28%</u>

Bolt and bearing capacities are accounting for double shear.
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	46.89 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
Max Drift, Δ_{MAX} =	0.938 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

3.4.14

$$L_b = 123 \text{ in}$$

$$J = 0.432$$

$$340.276$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 123$$

$$J = 0.432$$

$$216.395$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6$$

3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

Compression

3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

Compression

3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 18.1$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

3.4.14

$$\begin{aligned} L_b &= 63.42 \text{ in} \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \text{ ksi} \end{aligned}$$

Weak Axis:

3.4.14

$$\begin{aligned} L_b &= 63.42 \\ J &= 0.942 \\ &= 98.9729 \\ S1 &= \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left(\frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.2 \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.46712$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.7854$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 12.7711 \text{ ksi}$$

3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\
 \phi F_L &= 12.77 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 13.14 \text{ kips}
 \end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.9	-46.9	0	0
2	M14	Y	-46.9	-46.9	0	0
3	M15	Y	-46.9	-46.9	0	0
4	M16	Y	-46.9	-46.9	0	0

Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-48.164	-48.164	0	0
2	M14	y	-48.164	-48.164	0	0
3	M15	y	-74.435	-74.435	0	0
4	M16	y	-74.435	-74.435	0	0

Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	109.464	109.464	0	0
2	M14	y	83.192	83.192	0	0
3	M15	y	43.785	43.785	0	0
4	M16	y	43.785	43.785	0	0

Load Combinations

	Description	S... P...	S... B...	Fa... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...	B... Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8												
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6												
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6										
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3										
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3										
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25										
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25										



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 25° 100mph 30psf 10.25ft 7-05 NS.Pad] Page 19



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	114.157	1	221.616	1	-553	12	.013	2	-.007	15	.968	3
28			min	4.725	15	-314.854	3	-28.169	1	0	3	-.162	1	-.639	1
29		15	max	114.157	1	88.052	1	13.65	1	.013	2	-.007	12	1.216	3
30			min	4.725	15	-120.918	3	.576	15	0	3	-.171	1	-.816	1
31		16	max	114.157	1	73.019	3	55.469	1	.013	2	-.004	12	1.243	3
32			min	4.725	15	-45.511	1	2.296	15	0	3	-.131	1	-.84	1
33		17	max	114.157	1	266.955	3	97.287	1	.013	2	0	3	1.05	3
34			min	4.725	15	-179.074	1	4.015	15	0	3	-.044	1	-.712	1
35		18	max	114.157	1	460.892	3	139.106	1	.013	2	.09	1	.635	3
36			min	4.725	15	-312.637	1	5.735	15	0	3	.004	15	-.432	1
37		19	max	114.157	1	654.829	3	180.925	1	.013	2	.273	1	0	1
38			min	4.725	15	-446.2	1	7.455	15	0	3	.011	15	0	3
39	M14	1	max	52.206	1	470.915	1	-7.681	15	.008	3	.31	1	0	1
40			min	2.165	15	-506.443	3	-186.42	1	-.01	2	.013	15	0	3
41		2	max	52.206	1	337.352	1	-5.961	15	.008	3	.122	1	.494	3
42			min	2.165	15	-360.38	3	-144.602	1	-.01	2	.005	15	-.46	1
43		3	max	52.206	1	203.789	1	-4.241	15	.008	3	.001	3	.821	3
44			min	2.165	15	-214.317	3	-102.783	1	-.01	2	-.019	1	-.768	1
45		4	max	52.206	1	70.226	1	-2.522	15	.008	3	-.004	12	.982	3
46			min	2.165	15	-68.254	3	-60.964	1	-.01	2	-.113	1	-.924	1
47		5	max	52.206	1	77.809	3	-.802	15	.008	3	-.006	12	.976	3
48			min	2.165	15	-63.337	1	-19.146	1	-.01	2	-.158	1	-.928	1
49		6	max	52.206	1	223.872	3	22.673	1	.008	3	-.006	15	.805	3
50			min	2.165	15	-196.9	1	.336	12	-.01	2	-.156	1	-.78	1
51		7	max	52.206	1	369.936	3	64.492	1	.008	3	-.004	15	.466	3
52			min	2.165	15	-330.464	1	2.056	12	-.01	2	-.107	1	-.48	1
53		8	max	52.206	1	515.999	3	106.31	1	.008	3	0	10	0	15
54			min	2.165	15	-464.027	1	3.775	12	-.01	2	-.009	1	-.038	3
55		9	max	52.206	1	662.062	3	148.129	1	.008	3	.136	1	.577	1
56			min	2.165	15	-597.59	1	5.494	12	-.01	2	.003	12	-.709	3
57		10	max	52.206	1	731.153	1	-7.214	12	.008	3	.328	1	1.334	1
58			min	2.165	15	-808.125	3	-189.948	1	-.01	2	.01	12	-1.546	3
59		11	max	52.206	1	597.59	1	-5.494	12	.01	2	.136	1	.577	1
60			min	2.165	15	-662.062	3	-148.129	1	-.008	3	.003	12	-.709	3
61		12	max	52.206	1	464.027	1	-3.775	12	.01	2	0	10	0	15
62			min	2.165	15	-515.999	3	-106.31	1	-.008	3	-.009	1	-.038	3
63		13	max	52.206	1	330.464	1	-2.056	12	.01	2	-.004	15	.466	3
64			min	2.165	15	-369.936	3	-64.492	1	-.008	3	-.107	1	-.48	1
65		14	max	52.206	1	196.9	1	-.336	12	.01	2	-.006	15	.805	3
66			min	2.165	15	-223.872	3	-22.673	1	-.008	3	-.156	1	-.78	1
67		15	max	52.206	1	63.337	1	19.146	1	.01	2	-.006	12	.976	3
68			min	2.165	15	-77.809	3	.802	15	-.008	3	-.158	1	-.928	1
69		16	max	52.206	1	68.254	3	60.964	1	.01	2	-.004	12	.982	3
70			min	2.165	15	-70.226	1	2.522	15	-.008	3	-.113	1	-.924	1
71		17	max	52.206	1	214.317	3	102.783	1	.01	2	.001	3	.821	3
72			min	2.165	15	-203.789	1	4.241	15	-.008	3	-.019	1	-.768	1
73		18	max	52.206	1	360.38	3	144.602	1	.01	2	.122	1	.494	3
74			min	2.165	15	-337.352	1	5.961	15	-.008	3	.005	15	-.46	1
75		19	max	52.206	1	506.443	3	186.42	1	.01	2	.31	1	0	1
76			min	2.165	15	-470.915	1	7.681	15	-.008	3	.013	15	0	3
77	M15	1	max	-2.28	15	601	2	-7.679	15	.01	2	.31	1	0	2
78			min	-54.944	1	-264.286	3	-186.393	1	-.007	3	.013	15	0	15
79		2	max	-2.28	15	428.932	2	-5.959	15	.01	2	.121	1	.259	3
80			min	-54.944	1	-190.031	3	-144.575	1	-.007	3	.005	15	-.586	2
81		3	max	-2.28	15	256.865	2	-4.24	15	.01	2	.001	3	.433	3
82			min	-54.944	1	-115.777	3	-102.756	1	-.007	3	-.019	1	-.977	2
83		4	max	-2.28	15	84.798	2	-2.52	15	.01	2	-.004	12	.522	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-54.944	1	-41.522	3	-60.937	1	-.007	3	-.113	1	-1.172	2
85		5	max	-2.28	15	32.733	3	-.8	15	.01	2	-.006	12	.527	3
86			min	-54.944	1	-87.269	2	-19.119	1	-.007	3	-.158	1	-1.17	2
87		6	max	-2.28	15	106.988	3	22.7	1	.01	2	-.006	15	.448	3
88			min	-54.944	1	-259.336	2	.382	12	-.007	3	-.156	1	-.973	2
89		7	max	-2.28	15	181.243	3	64.519	1	.01	2	-.004	15	.284	3
90			min	-54.944	1	-431.403	2	2.102	12	-.007	3	-.107	1	-.579	2
91		8	max	-2.28	15	255.498	3	106.337	1	.01	2	0	10	.035	3
92			min	-54.944	1	-603.471	2	3.821	12	-.007	3	-.009	1	-.007	9
93		9	max	-2.28	15	329.752	3	148.156	1	.01	2	.136	1	.795	2
94			min	-54.944	1	-775.538	2	5.541	12	-.007	3	.003	12	-.298	3
95		10	max	-2.28	15	947.605	2	-7.26	12	.01	2	.328	1	1.776	2
96			min	-54.944	1	-404.007	3	-189.975	1	-.007	3	.011	12	-.716	3
97		11	max	-2.28	15	775.538	2	-5.541	12	.007	3	.136	1	.795	2
98			min	-54.944	1	-329.752	3	-148.156	1	-.01	2	.003	12	-.298	3
99		12	max	-2.28	15	603.471	2	-3.821	12	.007	3	0	10	.035	3
100			min	-54.944	1	-255.498	3	-106.337	1	-.01	2	-.009	1	-.007	9
101		13	max	-2.28	15	431.403	2	-2.102	12	.007	3	-.004	15	.284	3
102			min	-54.944	1	-181.243	3	-64.519	1	-.01	2	-.107	1	-.579	2
103		14	max	-2.28	15	259.336	2	-.382	12	.007	3	-.006	15	.448	3
104			min	-54.944	1	-106.988	3	-22.7	1	-.01	2	-.156	1	-.973	2
105		15	max	-2.28	15	87.269	2	19.119	1	.007	3	-.006	12	.527	3
106			min	-54.944	1	-32.733	3	.8	15	-.01	2	-.158	1	-1.17	2
107		16	max	-2.28	15	41.522	3	60.937	1	.007	3	-.004	12	.522	3
108			min	-54.944	1	-84.798	2	2.52	15	-.01	2	-.113	1	-1.172	2
109		17	max	-2.28	15	115.777	3	102.756	1	.007	3	.001	3	.433	3
110			min	-54.944	1	-256.865	2	4.24	15	-.01	2	-.019	1	-.977	2
111		18	max	-2.28	15	190.031	3	144.575	1	.007	3	.121	1	.259	3
112			min	-54.944	1	-428.932	2	5.959	15	-.01	2	.005	15	-.586	2
113		19	max	-2.28	15	264.286	3	186.393	1	.007	3	.31	1	0	2
114			min	-54.944	1	-601	2	7.679	15	-.01	2	.013	15	0	15
115	M16	1	max	-5.051	15	576.735	2	-7.462	15	.011	1	.274	1	0	2
116			min	-121.869	1	-246.802	3	-181.164	1	-.01	3	.011	15	0	3
117		2	max	-5.051	15	404.668	2	-5.742	15	.011	1	.092	1	.239	3
118			min	-121.869	1	-172.547	3	-139.346	1	-.01	3	.004	15	-.559	2
119		3	max	-5.051	15	232.601	2	-4.022	15	.011	1	0	12	.393	3
120			min	-121.869	1	-98.293	3	-97.527	1	-.01	3	-.043	1	-.922	2
121		4	max	-5.051	15	60.534	2	-2.303	15	.011	1	-.005	12	.463	3
122			min	-121.869	1	-24.038	3	-55.708	1	-.01	3	-.131	1	-1.089	2
123		5	max	-5.051	15	50.217	3	-.583	15	.011	1	-.007	12	.448	3
124			min	-121.869	1	-111.533	2	-13.89	1	-.01	3	-.17	1	-1.06	2
125		6	max	-5.051	15	124.472	3	27.929	1	.011	1	-.007	15	.348	3
126			min	-121.869	1	-283.601	2	.706	12	-.01	3	-.162	1	-.835	2
127		7	max	-5.051	15	198.727	3	69.748	1	.011	1	-.004	15	.164	3
128			min	-121.869	1	-455.668	2	2.426	12	-.01	3	-.107	1	-.414	2
129		8	max	-5.051	15	272.981	3	111.566	1	.011	1	0	10	.203	2
130			min	-121.869	1	-627.735	2	4.145	12	-.01	3	-.003	1	-.104	3
131		9	max	-5.051	15	347.236	3	153.385	1	.011	1	.148	1	1.016	2
132			min	-121.869	1	-799.802	2	5.865	12	-.01	3	.004	12	-.458	3
133		10	max	-5.051	15	971.869	2	-7.584	12	.011	1	.346	1	2.025	2
134			min	-121.869	1	-421.491	3	-195.204	1	-.01	3	.012	12	-.895	3
135		11	max	-5.051	15	799.802	2	-5.865	12	.01	3	.148	1	1.016	2
136			min	-121.869	1	-347.236	3	-153.385	1	-.011	1	.004	12	-.458	3
137		12	max	-5.051	15	627.735	2	-4.145	12	.01	3	0	10	.203	2
138			min	-121.869	1	-272.981	3	-111.566	1	-.011	1	-.003	1	-.104	3
139		13	max	-5.051	15	455.668	2	-2.426	12	.01	3	-.004	15	.164	3
140			min	-121.869	1	-198.727	3	-69.748	1	-.011	1	-.107	1	-.414	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-5.051	15	283.601	2	-.706	12	.01	3	-.007	15	.348	3
142			min	-121.869	1	-124.472	3	-27.929	1	-.011	1	-.162	1	-.835	2
143		15	max	-5.051	15	111.533	2	13.89	1	.01	3	-.007	12	.448	3
144			min	-121.869	1	-50.217	3	.583	15	-.011	1	-.17	1	-1.06	2
145		16	max	-5.051	15	24.038	3	55.708	1	.01	3	-.005	12	.463	3
146			min	-121.869	1	-60.534	2	2.303	15	-.011	1	-.131	1	-1.089	2
147		17	max	-5.051	15	98.293	3	97.527	1	.01	3	0	12	.393	3
148			min	-121.869	1	-232.601	2	4.022	15	-.011	1	-.043	1	-.922	2
149		18	max	-5.051	15	172.547	3	139.346	1	.01	3	.092	1	.239	3
150			min	-121.869	1	-404.668	2	5.742	15	-.011	1	.004	15	-.559	2
151		19	max	-5.051	15	246.802	3	181.164	1	.01	3	.274	1	0	2
152			min	-121.869	1	-576.735	2	7.462	15	-.011	1	.011	15	0	3
153	M2	1	max	1004.261	1	1.921	4	.736	1	0	5	0	3	0	1
154			min	-1183.035	3	.452	15	.03	15	0	1	0	1	0	1
155		2	max	1004.689	1	1.864	4	.736	1	0	5	0	1	0	15
156			min	-1182.714	3	.439	15	.03	15	0	1	0	15	0	4
157		3	max	1005.117	1	1.807	4	.736	1	0	5	0	1	0	15
158			min	-1182.393	3	.426	15	.03	15	0	1	0	15	-.001	4
159		4	max	1005.546	1	1.75	4	.736	1	0	5	0	1	0	15
160			min	-1182.071	3	.412	15	.03	15	0	1	0	15	-.002	4
161		5	max	1005.974	1	1.694	4	.736	1	0	5	0	1	0	15
162			min	-1181.75	3	.399	15	.03	15	0	1	0	15	-.002	4
163		6	max	1006.403	1	1.637	4	.736	1	0	5	.001	1	0	15
164			min	-1181.429	3	.386	15	.03	15	0	1	0	15	-.003	4
165		7	max	1006.831	1	1.58	4	.736	1	0	5	.001	1	0	15
166			min	-1181.107	3	.372	15	.03	15	0	1	0	15	-.003	4
167		8	max	1007.26	1	1.523	4	.736	1	0	5	.001	1	0	15
168			min	-1180.786	3	.359	15	.03	15	0	1	0	15	-.004	4
169		9	max	1007.688	1	1.466	4	.736	1	0	5	.002	1	0	15
170			min	-1180.465	3	.346	15	.03	15	0	1	0	15	-.004	4
171		10	max	1008.117	1	1.41	4	.736	1	0	5	.002	1	-.001	15
172			min	-1180.143	3	.332	15	.03	15	0	1	0	15	-.004	4
173		11	max	1008.545	1	1.353	4	.736	1	0	5	.002	1	-.001	15
174			min	-1179.822	3	.319	15	.03	15	0	1	0	15	-.005	4
175		12	max	1008.974	1	1.296	4	.736	1	0	5	.002	1	-.001	15
176			min	-1179.5	3	.305	15	.03	15	0	1	0	15	-.005	4
177		13	max	1009.402	1	1.239	4	.736	1	0	5	.003	1	-.001	15
178			min	-1179.179	3	.292	15	.03	15	0	1	0	15	-.006	4
179		14	max	1009.831	1	1.182	4	.736	1	0	5	.003	1	-.001	15
180			min	-1178.858	3	.273	12	.03	15	0	1	0	15	-.006	4
181		15	max	1010.259	1	1.126	4	.736	1	0	5	.003	1	-.001	15
182			min	-1178.536	3	.251	12	.03	15	0	1	0	15	-.006	4
183		16	max	1010.688	1	1.069	4	.736	1	0	5	.003	1	-.002	15
184			min	-1178.215	3	.229	12	.03	15	0	1	0	15	-.007	4
185		17	max	1011.116	1	1.012	4	.736	1	0	5	.003	1	-.002	15
186			min	-1177.894	3	.207	12	.03	15	0	1	0	15	-.007	4
187		18	max	1011.545	1	.965	2	.736	1	0	5	.004	1	-.002	15
188			min	-1177.572	3	.184	12	.03	15	0	1	0	15	-.007	4
189		19	max	1011.973	1	.921	2	.736	1	0	5	.004	1	-.002	15
190			min	-1177.251	3	.162	12	.03	15	0	1	0	15	-.007	4
191	M3	1	max	511.408	2	7.882	4	.167	1	0	5	0	1	.007	4
192			min	-653.922	3	1.853	15	.007	15	0	1	0	15	.002	15
193		2	max	511.237	2	7.115	4	.167	1	0	5	0	1	.004	2
194			min	-654.05	3	1.673	15	.007	15	0	1	0	15	0	12
195		3	max	511.067	2	6.347	4	.167	1	0	5	0	1	.002	2
196			min	-654.178	3	1.493	15	.007	15	0	1	0	15	0	3
197		4	max	510.897	2	5.58	4	.167	1	0	5	0	1	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-654.306	3	1.312	15	.007	15	0	1	0	15	-.002	3
199		5	max	510.726	2	4.813	4	.167	1	0	5	0	1	0	15
200			min	-654.433	3	1.132	15	.007	15	0	1	0	15	-.003	4
201		6	max	510.556	2	4.046	4	.167	1	0	5	0	1	-.001	15
202			min	-654.561	3	.952	15	.007	15	0	1	0	15	-.005	4
203		7	max	510.386	2	3.278	4	.167	1	0	5	0	1	-.002	15
204			min	-654.689	3	.771	15	.007	15	0	1	0	15	-.007	4
205		8	max	510.215	2	2.511	4	.167	1	0	5	0	1	-.002	15
206			min	-654.817	3	.591	15	.007	15	0	1	0	15	-.008	4
207		9	max	510.045	2	1.744	4	.167	1	0	5	0	1	-.002	15
208			min	-654.945	3	.411	15	.007	15	0	1	0	15	-.009	4
209		10	max	509.875	2	.977	4	.167	1	0	5	.001	1	-.002	15
210			min	-655.072	3	.23	15	.007	15	0	1	0	15	-.009	4
211		11	max	509.704	2	.316	2	.167	1	0	5	.001	1	-.002	15
212			min	-655.2	3	-.086	3	.007	15	0	1	0	15	-.01	4
213		12	max	509.534	2	-.131	15	.167	1	0	5	.001	1	-.002	15
214			min	-655.328	3	-.558	4	.007	15	0	1	0	15	-.01	4
215		13	max	509.364	2	-.311	15	.167	1	0	5	.001	1	-.002	15
216			min	-655.456	3	-1.325	4	.007	15	0	1	0	15	-.009	4
217		14	max	509.193	2	-.491	15	.167	1	0	5	.001	1	-.002	15
218			min	-655.583	3	-2.092	4	.007	15	0	1	0	15	-.008	4
219		15	max	509.023	2	-.672	15	.167	1	0	5	.001	1	-.002	15
220			min	-655.711	3	-2.859	4	.007	15	0	1	0	15	-.007	4
221		16	max	508.853	2	-.852	15	.167	1	0	5	.001	1	-.001	15
222			min	-655.839	3	-3.627	4	.007	15	0	1	0	15	-.006	4
223		17	max	508.682	2	-1.032	15	.167	1	0	5	.002	1	-.001	15
224			min	-655.967	3	-4.394	4	.007	15	0	1	0	15	-.004	4
225		18	max	508.512	2	-1.213	15	.167	1	0	5	.002	1	0	15
226			min	-656.094	3	-5.161	4	.007	15	0	1	0	15	-.002	4
227		19	max	508.342	2	-1.393	15	.167	1	0	5	.002	1	0	1
228			min	-656.222	3	-5.928	4	.007	15	0	1	0	15	0	1
229	M4	1	max	1130.578	1	0	1	-.479	15	0	1	.001	1	0	1
230			min	-115.503	3	0	1	-11.601	1	0	1	0	15	0	1
231		2	max	1130.748	1	0	1	-.479	15	0	1	0	3	0	1
232			min	-115.375	3	0	1	-11.601	1	0	1	0	1	0	1
233		3	max	1130.919	1	0	1	-.479	15	0	1	0	15	0	1
234			min	-115.248	3	0	1	-11.601	1	0	1	-.001	1	0	1
235		4	max	1131.089	1	0	1	-.479	15	0	1	0	15	0	1
236			min	-115.12	3	0	1	-11.601	1	0	1	-.003	1	0	1
237		5	max	1131.259	1	0	1	-.479	15	0	1	0	15	0	1
238			min	-114.992	3	0	1	-11.601	1	0	1	-.004	1	0	1
239		6	max	1131.43	1	0	1	-.479	15	0	1	0	15	0	1
240			min	-114.864	3	0	1	-11.601	1	0	1	-.005	1	0	1
241		7	max	1131.6	1	0	1	-.479	15	0	1	0	15	0	1
242			min	-114.737	3	0	1	-11.601	1	0	1	-.007	1	0	1
243		8	max	1131.77	1	0	1	-.479	15	0	1	0	15	0	1
244			min	-114.609	3	0	1	-11.601	1	0	1	-.008	1	0	1
245		9	max	1131.941	1	0	1	-.479	15	0	1	0	15	0	1
246			min	-114.481	3	0	1	-11.601	1	0	1	-.009	1	0	1
247		10	max	1132.111	1	0	1	-.479	15	0	1	0	15	0	1
248			min	-114.353	3	0	1	-11.601	1	0	1	-.011	1	0	1
249		11	max	1132.281	1	0	1	-.479	15	0	1	0	15	0	1
250			min	-114.225	3	0	1	-11.601	1	0	1	-.012	1	0	1
251		12	max	1132.452	1	0	1	-.479	15	0	1	0	15	0	1
252			min	-114.098	3	0	1	-11.601	1	0	1	-.013	1	0	1
253		13	max	1132.622	1	0	1	-.479	15	0	1	0	15	0	1
254			min	-113.97	3	0	1	-11.601	1	0	1	-.015	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1132.792	1	0	1	-479	15	0	1	0	15	0	1
256		min	-113.842	3	0	1	-11.601	1	0	1	-.016	1	0	1
257	15	max	1132.963	1	0	1	-479	15	0	1	0	15	0	1
258		min	-113.714	3	0	1	-11.601	1	0	1	-.017	1	0	1
259	16	max	1133.133	1	0	1	-479	15	0	1	0	15	0	1
260		min	-113.587	3	0	1	-11.601	1	0	1	-.019	1	0	1
261	17	max	1133.303	1	0	1	-479	15	0	1	0	15	0	1
262		min	-113.459	3	0	1	-11.601	1	0	1	-.02	1	0	1
263	18	max	1133.474	1	0	1	-479	15	0	1	0	15	0	1
264		min	-113.331	3	0	1	-11.601	1	0	1	-.021	1	0	1
265	19	max	1133.644	1	0	1	-479	15	0	1	0	15	0	1
266		min	-113.203	3	0	1	-11.601	1	0	1	-.023	1	0	1
267	M6	1	max	3234.985	1	2.299	2	0	1	0	0	1	0	1
268		min	-3876.324	3	.143	12	0	1	0	1	0	1	0	1
269	2	max	3235.414	1	2.254	2	0	1	0	1	0	1	0	12
270		min	-3876.003	3	.12	3	0	1	0	1	0	1	0	2
271	3	max	3235.842	1	2.21	2	0	1	0	1	0	1	0	3
272		min	-3875.681	3	.087	3	0	1	0	1	0	1	-.001	2
273	4	max	3236.271	1	2.166	2	0	1	0	1	0	1	0	3
274		min	-3875.36	3	.054	3	0	1	0	1	0	1	-.002	2
275	5	max	3236.699	1	2.122	2	0	1	0	1	0	1	0	3
276		min	-3875.038	3	.021	3	0	1	0	1	0	1	-.003	2
277	6	max	3237.128	1	2.077	2	0	1	0	1	0	1	0	3
278		min	-3874.717	3	-.013	3	0	1	0	1	0	1	-.003	2
279	7	max	3237.556	1	2.033	2	0	1	0	1	0	1	0	3
280		min	-3874.396	3	-.046	3	0	1	0	1	0	1	-.004	2
281	8	max	3237.985	1	1.989	2	0	1	0	1	0	1	0	3
282		min	-3874.074	3	-.079	3	0	1	0	1	0	1	-.004	2
283	9	max	3238.413	1	1.945	2	0	1	0	1	0	1	0	3
284		min	-3873.753	3	-.112	3	0	1	0	1	0	1	-.005	2
285	10	max	3238.842	1	1.9	2	0	1	0	1	0	1	0	3
286		min	-3873.432	3	-.145	3	0	1	0	1	0	1	-.005	2
287	11	max	3239.27	1	1.856	2	0	1	0	1	0	1	0	3
288		min	-3873.11	3	-.178	3	0	1	0	1	0	1	-.006	2
289	12	max	3239.699	1	1.812	2	0	1	0	1	0	1	0	3
290		min	-3872.789	3	-.212	3	0	1	0	1	0	1	-.007	2
291	13	max	3240.127	1	1.768	2	0	1	0	1	0	1	0	3
292		min	-3872.467	3	-.245	3	0	1	0	1	0	1	-.007	2
293	14	max	3240.556	1	1.723	2	0	1	0	1	0	1	0	3
294		min	-3872.146	3	-.278	3	0	1	0	1	0	1	-.008	2
295	15	max	3240.984	1	1.679	2	0	1	0	1	0	1	0	3
296		min	-3871.825	3	-.311	3	0	1	0	1	0	1	-.008	2
297	16	max	3241.413	1	1.635	2	0	1	0	1	0	1	0	3
298		min	-3871.503	3	-.344	3	0	1	0	1	0	1	-.009	2
299	17	max	3241.841	1	1.591	2	0	1	0	1	0	1	0	3
300		min	-3871.182	3	-.378	3	0	1	0	1	0	1	-.009	2
301	18	max	3242.27	1	1.546	2	0	1	0	1	0	1	0	3
302		min	-3870.861	3	-.411	3	0	1	0	1	0	1	-.009	2
303	19	max	3242.698	1	1.502	2	0	1	0	1	0	1	0	3
304		min	-3870.539	3	-.444	3	0	1	0	1	0	1	-.01	2
305	M7	1	max	1980.948	2	7.918	4	0	1	0	0	1	.01	2
306		min	-2059.842	3	1.859	15	0	1	0	1	0	1	0	3
307	2	max	1980.778	2	7.151	4	0	1	0	1	0	1	.007	2
308		min	-2059.97	3	1.678	15	0	1	0	1	0	1	-.002	3
309	3	max	1980.608	2	6.384	4	0	1	0	1	0	1	.005	2
310		min	-2060.097	3	1.498	15	0	1	0	1	0	1	-.004	3
311	4	max	1980.437	2	5.617	4	0	1	0	1	0	1	.003	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2060.225	3	1.318	15	0	1	0	1	0	1	-.005	3
313	5	max	1980.267	2	4.85	4	0	1	0	1	0	1	0	2
314		min	-2060.353	3	1.137	15	0	1	0	1	0	1	-.006	3
315	6	max	1980.096	2	4.082	4	0	1	0	1	0	1	0	2
316		min	-2060.481	3	.957	15	0	1	0	1	0	1	-.007	3
317	7	max	1979.926	2	3.315	4	0	1	0	1	0	1	-.002	15
318		min	-2060.608	3	.777	15	0	1	0	1	0	1	-.007	3
319	8	max	1979.756	2	2.548	4	0	1	0	1	0	1	-.002	15
320		min	-2060.736	3	.559	12	0	1	0	1	0	1	-.008	4
321	9	max	1979.585	2	1.915	2	0	1	0	1	0	1	-.002	15
322		min	-2060.864	3	.26	12	0	1	0	1	0	1	-.009	4
323	10	max	1979.415	2	1.317	2	0	1	0	1	0	1	-.002	15
324		min	-2060.992	3	-.101	3	0	1	0	1	0	1	-.009	4
325	11	max	1979.245	2	.719	2	0	1	0	1	0	1	-.002	15
326		min	-2061.119	3	-.549	3	0	1	0	1	0	1	-.009	4
327	12	max	1979.074	2	.121	2	0	1	0	1	0	1	-.002	15
328		min	-2061.247	3	-.997	3	0	1	0	1	0	1	-.009	4
329	13	max	1978.904	2	-.306	15	0	1	0	1	0	1	-.002	15
330		min	-2061.375	3	-1.446	3	0	1	0	1	0	1	-.009	4
331	14	max	1978.734	2	-.486	15	0	1	0	1	0	1	-.002	15
332		min	-2061.503	3	-2.055	4	0	1	0	1	0	1	-.008	4
333	15	max	1978.563	2	-.666	15	0	1	0	1	0	1	-.002	15
334		min	-2061.63	3	-2.823	4	0	1	0	1	0	1	-.007	4
335	16	max	1978.393	2	-.847	15	0	1	0	1	0	1	-.001	15
336		min	-2061.758	3	-3.59	4	0	1	0	1	0	1	-.006	4
337	17	max	1978.223	2	-1.027	15	0	1	0	1	0	1	-.001	15
338		min	-2061.886	3	-4.357	4	0	1	0	1	0	1	-.004	4
339	18	max	1978.052	2	-1.207	15	0	1	0	1	0	1	0	15
340		min	-2062.014	3	-5.124	4	0	1	0	1	0	1	-.002	4
341	19	max	1977.882	2	-1.388	15	0	1	0	1	0	1	0	1
342		min	-2062.142	3	-5.892	4	0	1	0	1	0	1	0	1
343	M8	1	max	3054.119	1	0	1	0	1	0	1	0	1	1
344		min	-456.817	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3054.289	1	0	1	0	1	0	1	0	1	0	1
346		min	-456.689	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3054.46	1	0	1	0	1	0	1	0	1	0	1
348		min	-456.562	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3054.63	1	0	1	0	1	0	1	0	1	0	1
350		min	-456.434	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3054.8	1	0	1	0	1	0	1	0	1	0	1
352		min	-456.306	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3054.971	1	0	1	0	1	0	1	0	1	0	1
354		min	-456.178	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3055.141	1	0	1	0	1	0	1	0	1	0	1
356		min	-456.051	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3055.311	1	0	1	0	1	0	1	0	1	0	1
358		min	-455.923	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3055.482	1	0	1	0	1	0	1	0	1	0	1
360		min	-455.795	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3055.652	1	0	1	0	1	0	1	0	1	0	1
362		min	-455.667	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3055.822	1	0	1	0	1	0	1	0	1	0	1
364		min	-455.54	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3055.993	1	0	1	0	1	0	1	0	1	0	1
366		min	-455.412	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3056.163	1	0	1	0	1	0	1	0	1	0	1
368		min	-455.284	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3056.333	1	0	1	0	1	0	1	0	1	0	1
370			min	-455.156	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3056.504	1	0	1	0	1	0	1	0	1	0	1
372			min	-455.029	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3056.674	1	0	1	0	1	0	1	0	1	0	1
374			min	-454.901	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3056.844	1	0	1	0	1	0	1	0	1	0	1
376			min	-454.773	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3057.015	1	0	1	0	1	0	1	0	1	0	1
378			min	-454.645	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3057.185	1	0	1	0	1	0	1	0	1	0	1
380			min	-454.517	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1004.261	1	1.921	4	-.03	15	0	1	0	1	0	1
382			min	-1183.035	3	.452	15	-.736	1	0	5	0	3	0	1
383		2	max	1004.689	1	1.864	4	-.03	15	0	1	0	15	0	15
384			min	-1182.714	3	.439	15	-.736	1	0	5	0	1	0	4
385		3	max	1005.117	1	1.807	4	-.03	15	0	1	0	15	0	15
386			min	-1182.393	3	.426	15	-.736	1	0	5	0	1	-.001	4
387		4	max	1005.546	1	1.75	4	-.03	15	0	1	0	15	0	15
388			min	-1182.071	3	.412	15	-.736	1	0	5	0	1	-.002	4
389		5	max	1005.974	1	1.694	4	-.03	15	0	1	0	15	0	15
390			min	-1181.75	3	.399	15	-.736	1	0	5	0	1	-.002	4
391		6	max	1006.403	1	1.637	4	-.03	15	0	1	0	15	0	15
392			min	-1181.429	3	.386	15	-.736	1	0	5	-.001	1	-.003	4
393		7	max	1006.831	1	1.58	4	-.03	15	0	1	0	15	0	15
394			min	-1181.107	3	.372	15	-.736	1	0	5	-.001	1	-.003	4
395		8	max	1007.26	1	1.523	4	-.03	15	0	1	0	15	0	15
396			min	-1180.786	3	.359	15	-.736	1	0	5	-.001	1	-.004	4
397		9	max	1007.688	1	1.466	4	-.03	15	0	1	0	15	0	15
398			min	-1180.465	3	.346	15	-.736	1	0	5	-.002	1	-.004	4
399		10	max	1008.117	1	1.41	4	-.03	15	0	1	0	15	-.001	15
400			min	-1180.143	3	.332	15	-.736	1	0	5	-.002	1	-.004	4
401		11	max	1008.545	1	1.353	4	-.03	15	0	1	0	15	-.001	15
402			min	-1179.822	3	.319	15	-.736	1	0	5	-.002	1	-.005	4
403		12	max	1008.974	1	1.296	4	-.03	15	0	1	0	15	-.001	15
404			min	-1179.5	3	.305	15	-.736	1	0	5	-.002	1	-.005	4
405		13	max	1009.402	1	1.239	4	-.03	15	0	1	0	15	-.001	15
406			min	-1179.179	3	.292	15	-.736	1	0	5	-.003	1	-.006	4
407		14	max	1009.831	1	1.182	4	-.03	15	0	1	0	15	-.001	15
408			min	-1178.858	3	.273	12	-.736	1	0	5	-.003	1	-.006	4
409		15	max	1010.259	1	1.126	4	-.03	15	0	1	0	15	-.001	15
410			min	-1178.536	3	.251	12	-.736	1	0	5	-.003	1	-.006	4
411		16	max	1010.688	1	1.069	4	-.03	15	0	1	0	15	-.002	15
412			min	-1178.215	3	.229	12	-.736	1	0	5	-.003	1	-.007	4
413		17	max	1011.116	1	1.012	4	-.03	15	0	1	0	15	-.002	15
414			min	-1177.894	3	.207	12	-.736	1	0	5	-.003	1	-.007	4
415		18	max	1011.545	1	.965	2	-.03	15	0	1	0	15	-.002	15
416			min	-1177.572	3	.184	12	-.736	1	0	5	-.004	1	-.007	4
417		19	max	1011.973	1	.921	2	-.03	15	0	1	0	15	-.002	15
418			min	-1177.251	3	.162	12	-.736	1	0	5	-.004	1	-.007	4
419	M11	1	max	511.408	2	7.882	4	-.007	15	0	1	0	15	.007	4
420			min	-653.922	3	1.853	15	-.167	1	0	5	0	1	.002	15
421		2	max	511.237	2	7.115	4	-.007	15	0	1	0	15	.004	2
422			min	-654.05	3	1.673	15	-.167	1	0	5	0	1	0	12
423		3	max	511.067	2	6.347	4	-.007	15	0	1	0	15	.002	2
424			min	-654.178	3	1.493	15	-.167	1	0	5	0	1	0	3
425		4	max	510.897	2	5.58	4	-.007	15	0	1	0	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-654.306	3	1.312	15	-.167	1	0	5	0	1	-.002	3
427		5	max	510.726	2	4.813	4	-.007	15	0	1	0	15	0	15
428			min	-654.433	3	1.132	15	-.167	1	0	5	0	1	-.003	4
429		6	max	510.556	2	4.046	4	-.007	15	0	1	0	15	-.001	15
430			min	-654.561	3	.952	15	-.167	1	0	5	0	1	-.005	4
431		7	max	510.386	2	3.278	4	-.007	15	0	1	0	15	-.002	15
432			min	-654.689	3	.771	15	-.167	1	0	5	0	1	-.007	4
433		8	max	510.215	2	2.511	4	-.007	15	0	1	0	15	-.002	15
434			min	-654.817	3	.591	15	-.167	1	0	5	0	1	-.008	4
435		9	max	510.045	2	1.744	4	-.007	15	0	1	0	15	-.002	15
436			min	-654.945	3	.411	15	-.167	1	0	5	0	1	-.009	4
437		10	max	509.875	2	.977	4	-.007	15	0	1	0	15	-.002	15
438			min	-655.072	3	.23	15	-.167	1	0	5	-.001	1	-.009	4
439		11	max	509.704	2	.316	2	-.007	15	0	1	0	15	-.002	15
440			min	-655.2	3	-.086	3	-.167	1	0	5	-.001	1	-.01	4
441		12	max	509.534	2	-.131	15	-.007	15	0	1	0	15	-.002	15
442			min	-655.328	3	-.558	4	-.167	1	0	5	-.001	1	-.01	4
443		13	max	509.364	2	-.311	15	-.007	15	0	1	0	15	-.002	15
444			min	-655.456	3	-1.325	4	-.167	1	0	5	-.001	1	-.009	4
445		14	max	509.193	2	-.491	15	-.007	15	0	1	0	15	-.002	15
446			min	-655.583	3	-2.092	4	-.167	1	0	5	-.001	1	-.008	4
447		15	max	509.023	2	-.672	15	-.007	15	0	1	0	15	-.002	15
448			min	-655.711	3	-2.859	4	-.167	1	0	5	-.001	1	-.007	4
449		16	max	508.853	2	-.852	15	-.007	15	0	1	0	15	-.001	15
450			min	-655.839	3	-3.627	4	-.167	1	0	5	-.001	1	-.006	4
451		17	max	508.682	2	-1.032	15	-.007	15	0	1	0	15	-.001	15
452			min	-655.967	3	-4.394	4	-.167	1	0	5	-.002	1	-.004	4
453		18	max	508.512	2	-1.213	15	-.007	15	0	1	0	15	0	15
454			min	-656.094	3	-5.161	4	-.167	1	0	5	-.002	1	-.002	4
455		19	max	508.342	2	-1.393	15	-.007	15	0	1	0	15	0	1
456			min	-656.222	3	-5.928	4	-.167	1	0	5	-.002	1	0	1
457	M12	1	max	1130.578	1	0	1	11.601	1	0	1	0	15	0	1
458			min	-115.503	3	0	1	.479	15	0	1	-.001	1	0	1
459		2	max	1130.748	1	0	1	11.601	1	0	1	0	1	0	1
460			min	-115.375	3	0	1	.479	15	0	1	0	3	0	1
461		3	max	1130.919	1	0	1	11.601	1	0	1	.001	1	0	1
462			min	-115.248	3	0	1	.479	15	0	1	0	15	0	1
463		4	max	1131.089	1	0	1	11.601	1	0	1	.003	1	0	1
464			min	-115.12	3	0	1	.479	15	0	1	0	15	0	1
465		5	max	1131.259	1	0	1	11.601	1	0	1	.004	1	0	1
466			min	-114.992	3	0	1	.479	15	0	1	0	15	0	1
467		6	max	1131.43	1	0	1	11.601	1	0	1	.005	1	0	1
468			min	-114.864	3	0	1	.479	15	0	1	0	15	0	1
469		7	max	1131.6	1	0	1	11.601	1	0	1	.007	1	0	1
470			min	-114.737	3	0	1	.479	15	0	1	0	15	0	1
471		8	max	1131.77	1	0	1	11.601	1	0	1	.008	1	0	1
472			min	-114.609	3	0	1	.479	15	0	1	0	15	0	1
473		9	max	1131.941	1	0	1	11.601	1	0	1	.009	1	0	1
474			min	-114.481	3	0	1	.479	15	0	1	0	15	0	1
475		10	max	1132.111	1	0	1	11.601	1	0	1	.011	1	0	1
476			min	-114.353	3	0	1	.479	15	0	1	0	15	0	1
477		11	max	1132.281	1	0	1	11.601	1	0	1	.012	1	0	1
478			min	-114.225	3	0	1	.479	15	0	1	0	15	0	1
479		12	max	1132.452	1	0	1	11.601	1	0	1	.013	1	0	1
480			min	-114.098	3	0	1	.479	15	0	1	0	15	0	1
481		13	max	1132.622	1	0	1	11.601	1	0	1	.015	1	0	1
482			min	-113.97	3	0	1	.479	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1132.792	1	0	1	11.601	1	0	1	.016	1	0	1
484		min	-113.842	3	0	1	.479	15	0	1	0	15	0	1
485	15	max	1132.963	1	0	1	11.601	1	0	1	.017	1	0	1
486		min	-113.714	3	0	1	.479	15	0	1	0	15	0	1
487	16	max	1133.133	1	0	1	11.601	1	0	1	.019	1	0	1
488		min	-113.587	3	0	1	.479	15	0	1	0	15	0	1
489	17	max	1133.303	1	0	1	11.601	1	0	1	.02	1	0	1
490		min	-113.459	3	0	1	.479	15	0	1	0	15	0	1
491	18	max	1133.474	1	0	1	11.601	1	0	1	.021	1	0	1
492		min	-113.331	3	0	1	.479	15	0	1	0	15	0	1
493	19	max	1133.644	1	0	1	11.601	1	0	1	.023	1	0	1
494		min	-113.203	3	0	1	.479	15	0	1	0	15	0	1
495	M1	1	max	180.931	1	654.806	3	-4.725	15	0	.273	1	0	3
496		min	7.455	15	-444.807	1	-114.013	1	0	3	.011	15	-.013	2
497	2	max	181.536	1	653.832	3	-4.725	15	0	1	.212	1	.223	1
498		min	7.638	15	-446.105	1	-114.013	1	0	3	.009	15	-.345	3
499	3	max	400.818	3	499.221	1	-4.695	15	0	3	.152	1	.447	1
500		min	-237.603	2	-466.957	3	-113.525	1	0	1	.006	15	-.676	3
501	4	max	401.272	3	497.922	1	-4.695	15	0	3	.092	1	.184	1
502		min	-236.997	2	-467.931	3	-113.525	1	0	1	.004	15	-.43	3
503	5	max	401.726	3	496.624	1	-4.695	15	0	3	.032	1	-.003	15
504		min	-236.392	2	-468.905	3	-113.525	1	0	1	.001	15	-.183	3
505	6	max	402.18	3	495.326	1	-4.695	15	0	3	-.001	15	.065	3
506		min	-235.786	2	-469.878	3	-113.525	1	0	1	-.027	1	-.352	2
507	7	max	402.634	3	494.028	1	-4.695	15	0	3	-.004	15	.313	3
508		min	-235.181	2	-470.852	3	-113.525	1	0	1	-.087	1	-.606	2
509	8	max	403.088	3	492.729	1	-4.695	15	0	3	-.006	15	.562	3
510		min	-234.576	2	-471.826	3	-113.525	1	0	1	-.147	1	-.861	1
511	9	max	415.991	3	40.949	2	-6.859	15	0	9	.086	1	.658	3
512		min	-158.81	2	.395	15	-165.768	1	0	3	.004	15	-.983	2
513	10	max	416.445	3	39.651	2	-6.859	15	0	9	0	15	.639	3
514		min	-158.205	2	.004	15	-165.768	1	0	3	-.001	1	-1.004	2
515	11	max	416.899	3	38.353	2	-6.859	15	0	9	-.004	15	.621	3
516		min	-157.599	2	-1.585	4	-165.768	1	0	3	-.089	1	-1.025	2
517	12	max	429.729	3	299.642	3	-4.58	15	0	2	.145	1	.541	3
518		min	-98.126	10	-563.294	2	-110.887	1	0	3	.006	15	-.908	2
519	13	max	430.183	3	298.668	3	-4.58	15	0	2	.087	1	.383	3
520		min	-97.622	10	-564.592	2	-110.887	1	0	3	.004	15	-.61	2
521	14	max	430.638	3	297.695	3	-4.58	15	0	2	.028	1	.226	3
522		min	-97.117	10	-565.891	2	-110.887	1	0	3	.001	15	-.32	1
523	15	max	431.092	3	296.721	3	-4.58	15	0	2	-.001	15	.069	3
524		min	-96.613	10	-567.189	2	-110.887	1	0	3	-.03	1	-.036	1
525	16	max	431.546	3	295.747	3	-4.58	15	0	2	-.004	15	.287	2
526		min	-96.109	10	-568.487	2	-110.887	1	0	3	-.089	1	-.088	3
527	17	max	432	3	294.774	3	-4.58	15	0	2	-.006	15	.587	2
528		min	-95.604	10	-569.785	2	-110.887	1	0	3	-.147	1	-.243	3
529	18	max	-7.645	15	578.546	2	-5.051	15	0	3	-.009	15	.295	2
530		min	-181.765	1	-245.887	3	-122.009	1	0	2	-.21	1	-.12	3
531	19	max	-7.462	15	577.248	2	-5.051	15	0	3	-.011	15	.01	3
532		min	-181.16	1	-246.861	3	-122.009	1	0	2	-.274	1	-.011	1
533	M5	1	max	390.875	1	2181.125	3	0	1	0	0	1	.026	2
534		min	14.864	12	-1503.497	1	0	1	0	1	0	1	0	3
535	2	max	391.48	1	2180.151	3	0	1	0	1	0	1	.818	1
536		min	15.166	12	-1504.795	1	0	1	0	1	0	1	-1.151	3
537	3	max	1290.039	3	1526.356	1	0	1	0	1	0	1	1.576	1
538		min	-845.603	2	-1504.577	3	0	1	0	1	0	1	-2.257	3
539	4	max	1290.493	3	1525.058	1	0	1	0	1	0	1	.771	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-844.998	2	-1505.551	3	0	1	0	1	0	1	-1.463	3
541		5	max	1290.947	3	1523.759	1	0	1	0	1	0	1	.007	9
542			min	-844.393	2	-1506.524	3	0	1	0	1	0	1	-.668	3
543		6	max	1291.401	3	1522.461	1	0	1	0	1	0	1	.127	3
544			min	-843.787	2	-1507.498	3	0	1	0	1	0	1	-.866	2
545		7	max	1291.855	3	1521.163	1	0	1	0	1	0	1	.923	3
546			min	-843.182	2	-1508.472	3	0	1	0	1	0	1	-1.644	2
547		8	max	1292.309	3	1519.865	1	0	1	0	1	0	1	1.719	3
548			min	-842.576	2	-1509.445	3	0	1	0	1	0	1	-2.443	1
549		9	max	1314.292	3	136.311	2	0	1	0	1	0	1	1.982	3
550			min	-686.514	2	.393	15	0	1	0	1	0	1	-2.766	1
551		10	max	1314.746	3	135.012	2	0	1	0	1	0	1	1.916	3
552			min	-685.909	2	.001	15	0	1	0	1	0	1	-2.826	2
553		11	max	1315.2	3	133.714	2	0	1	0	1	0	1	1.85	3
554			min	-685.303	2	-1.436	4	0	1	0	1	0	1	-2.897	2
555		12	max	1337.328	3	953.863	3	0	1	0	1	0	1	1.624	3
556			min	-529.275	2	-1749.004	2	0	1	0	1	0	1	-2.592	2
557		13	max	1337.782	3	952.89	3	0	1	0	1	0	1	1.12	3
558			min	-528.669	2	-1750.302	2	0	1	0	1	0	1	-1.669	2
559		14	max	1338.236	3	951.916	3	0	1	0	1	0	1	.618	3
560			min	-528.064	2	-1751.6	2	0	1	0	1	0	1	-.775	1
561		15	max	1338.69	3	950.942	3	0	1	0	1	0	1	.18	2
562			min	-527.459	2	-1752.898	2	0	1	0	1	0	1	-.004	13
563		16	max	1339.144	3	949.969	3	0	1	0	1	0	1	1.105	2
564			min	-526.853	2	-1754.197	2	0	1	0	1	0	1	-.386	3
565		17	max	1339.598	3	948.995	3	0	1	0	1	0	1	2.031	2
566			min	-526.248	2	-1755.495	2	0	1	0	1	0	1	-.887	3
567		18	max	-15.471	12	1947.978	2	0	1	0	1	0	1	1.047	2
568			min	-391.022	1	-842.366	3	0	1	0	1	0	1	-.464	3
569		19	max	-15.168	12	1946.68	2	0	1	0	1	0	1	.021	1
570			min	-390.417	1	-843.34	3	0	1	0	1	0	1	-.019	3
571	M9	1	max	180.931	1	654.806	3	114.013	1	0	3	-.011	15	0	3
572			min	7.455	15	-444.807	1	4.725	15	0	1	-.273	1	-.013	2
573		2	max	181.536	1	653.832	3	114.013	1	0	3	-.009	15	.223	1
574			min	7.638	15	-446.105	1	4.725	15	0	1	-.212	1	-.345	3
575		3	max	400.818	3	499.221	1	113.525	1	0	1	-.006	15	.447	1
576			min	-237.603	2	-466.957	3	4.695	15	0	3	-.152	1	-.676	3
577		4	max	401.272	3	497.922	1	113.525	1	0	1	-.004	15	.184	1
578			min	-236.997	2	-467.931	3	4.695	15	0	3	-.092	1	-.43	3
579		5	max	401.726	3	496.624	1	113.525	1	0	1	-.001	15	-.003	15
580			min	-236.392	2	-468.905	3	4.695	15	0	3	-.032	1	-.183	3
581		6	max	402.18	3	495.326	1	113.525	1	0	1	.027	1	.065	3
582			min	-235.786	2	-469.878	3	4.695	15	0	3	.001	15	-.352	2
583		7	max	402.634	3	494.028	1	113.525	1	0	1	.087	1	.313	3
584			min	-235.181	2	-470.852	3	4.695	15	0	3	.004	15	-.606	2
585		8	max	403.088	3	492.729	1	113.525	1	0	1	.147	1	.562	3
586			min	-234.576	2	-471.826	3	4.695	15	0	3	.006	15	-.861	1
587		9	max	415.991	3	40.949	2	165.768	1	0	3	-.004	15	.658	3
588			min	-158.81	2	.395	15	6.859	15	0	9	-.086	1	-.983	2
589		10	max	416.445	3	39.651	2	165.768	1	0	3	.001	1	.639	3
590			min	-158.205	2	.004	15	6.859	15	0	9	0	15	-1.004	2
591		11	max	416.899	3	38.353	2	165.768	1	0	3	.089	1	.621	3
592			min	-157.599	2	-1.585	4	6.859	15	0	9	.004	15	-1.025	2
593		12	max	429.729	3	299.642	3	110.887	1	0	3	-.006	15	.541	3
594			min	-98.126	10	-563.294	2	4.58	15	0	2	-.145	1	-.908	2
595		13	max	430.183	3	298.668	3	110.887	1	0	3	-.004	15	.383	3
596			min	-97.622	10	-564.592	2	4.58	15	0	2	-.087	1	-.61	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	430.638	3	297.695	3	110.887	1	0	3	-.001	15	.226	3
598		min	-97.117	10	-565.891	2	4.58	15	0	2	-.028	1	-.32	1
599	15	max	431.092	3	296.721	3	110.887	1	0	3	.03	1	.069	3
600		min	-96.613	10	-567.189	2	4.58	15	0	2	.001	15	-.036	1
601	16	max	431.546	3	295.747	3	110.887	1	0	3	.089	1	.287	2
602		min	-96.109	10	-568.487	2	4.58	15	0	2	.004	15	-.088	3
603	17	max	432	3	294.774	3	110.887	1	0	3	.147	1	.587	2
604		min	-95.604	10	-569.785	2	4.58	15	0	2	.006	15	-.243	3
605	18	max	-7.645	15	578.546	2	122.009	1	0	2	.21	1	.295	2
606		min	-181.765	1	-245.887	3	5.051	15	0	3	.009	15	-.12	3
607	19	max	-7.462	15	577.248	2	122.009	1	0	2	.274	1	.01	3
608		min	-181.16	1	-246.861	3	5.051	15	0	3	.011	15	-.011	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.102	2	.007	3	8.347e-3	2	NC	1	NC	1
2			min	0	15	-.012	3	-.003	2	-1.177e-3	3	NC	1	NC	1
3		2	max	0	1	.322	3	.045	1	9.63e-3	2	NC	5	NC	2
4			min	0	15	-.109	1	.002	10	-1.193e-3	3	736.14	3	5693.598	1
5		3	max	0	1	.592	3	.107	1	1.091e-2	2	NC	5	NC	3
6			min	0	15	-.273	1	.005	15	-1.209e-3	3	406.711	3	2322.459	1
7		4	max	0	1	.757	3	.162	1	1.22e-2	2	NC	5	NC	3
8			min	0	15	-.366	1	.007	15	-1.224e-3	3	319.742	3	1534.553	1
9		5	max	0	1	.795	3	.19	1	1.348e-2	2	NC	5	NC	3
10			min	0	15	-.373	1	.008	15	-1.24e-3	3	304.53	3	1305.932	1
11		6	max	0	1	.71	3	.184	1	1.476e-2	2	NC	5	NC	3
12			min	0	15	-.297	1	.008	15	-1.256e-3	3	340.27	3	1351.84	1
13		7	max	0	1	.528	3	.144	1	1.605e-2	2	NC	5	NC	3
14			min	0	15	-.157	1	.006	15	-1.272e-3	3	455.243	3	1722.377	1
15		8	max	0	1	.296	3	.084	1	1.733e-2	2	NC	4	NC	3
16			min	0	15	-.004	9	.001	10	-1.287e-3	3	797.858	3	2982.318	1
17		9	max	0	1	.178	2	.024	1	1.861e-2	2	NC	4	NC	1
18			min	0	15	.005	15	-.006	10	-1.303e-3	3	2514.615	3	NC	1
19		10	max	0	1	.242	2	.022	3	1.99e-2	2	NC	3	NC	1
20			min	0	1	-.01	3	-.015	2	-1.319e-3	3	1754.699	2	NC	1
21		11	max	0	15	.178	2	.024	1	1.861e-2	2	NC	4	NC	1
22			min	0	1	.005	15	-.006	10	-1.303e-3	3	2514.615	3	NC	1
23		12	max	0	15	.296	3	.084	1	1.733e-2	2	NC	4	NC	3
24			min	0	1	-.004	9	.001	10	-1.287e-3	3	797.858	3	2982.318	1
25		13	max	0	15	.528	3	.144	1	1.605e-2	2	NC	5	NC	3
26			min	0	1	-.157	1	.006	15	-1.272e-3	3	455.243	3	1722.377	1
27		14	max	0	15	.71	3	.184	1	1.476e-2	2	NC	5	NC	3
28			min	0	1	-.297	1	.008	15	-1.256e-3	3	340.27	3	1351.84	1
29		15	max	0	15	.795	3	.19	1	1.348e-2	2	NC	5	NC	3
30			min	0	1	-.373	1	.008	15	-1.24e-3	3	304.53	3	1305.932	1
31		16	max	0	15	.757	3	.162	1	1.22e-2	2	NC	5	NC	3
32			min	0	1	-.366	1	.007	15	-1.224e-3	3	319.742	3	1534.553	1
33		17	max	0	15	.592	3	.107	1	1.091e-2	2	NC	5	NC	3
34			min	0	1	-.273	1	.005	15	-1.209e-3	3	406.711	3	2322.459	1
35		18	max	0	15	.322	3	.045	1	9.63e-3	2	NC	5	NC	2
36			min	0	1	-.109	1	.002	10	-1.193e-3	3	736.14	3	5693.598	1
37		19	max	0	15	.102	2	.007	3	8.347e-3	2	NC	1	NC	1
38			min	-.001	1	-.012	3	-.003	2	-1.177e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.195	3	.006	3	4.974e-3	2	NC	1	NC	1
40			min	0	15	-.328	2	-.003	2	-3.502e-3	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.509	3	.031	1	5.976e-3	2	NC	5	NC	2
42		min	0	15	-.648	1	0	10	-4.276e-3	3	758.144	1	8323.993	1
43	3	max	0	1	.774	3	.087	1	6.977e-3	2	NC	15	NC	3
44		min	0	15	-.926	1	.004	15	-5.051e-3	3	408.039	1	2891.683	1
45	4	max	0	1	.956	3	.139	1	7.979e-3	2	NC	15	NC	3
46		min	0	15	-1.127	1	.006	15	-5.825e-3	3	306.159	1	1790.124	1
47	5	max	0	1	1.039	3	.169	1	8.98e-3	2	NC	15	NC	3
48		min	0	15	-1.234	1	.007	15	-6.599e-3	3	270.247	1	1470.273	1
49	6	max	0	1	1.022	3	.167	1	9.982e-3	2	9990.072	15	NC	3
50		min	0	15	-1.246	1	.007	15	-7.373e-3	3	266.58	1	1488	1
51	7	max	0	1	.924	3	.134	1	1.098e-2	2	NC	15	NC	3
52		min	0	15	-1.18	1	.006	15	-8.148e-3	3	287.274	1	1865.668	1
53	8	max	0	1	.779	3	.079	1	1.198e-2	2	NC	15	NC	2
54		min	0	15	-1.065	1	.001	10	-8.922e-3	3	331.642	1	3187.26	1
55	9	max	0	1	.64	3	.023	1	1.299e-2	2	NC	15	NC	1
56		min	0	15	-.952	2	-.006	10	-9.696e-3	3	393.173	1	NC	1
57	10	max	0	1	.575	3	.02	3	1.399e-2	2	NC	5	NC	1
58		min	0	1	-.901	2	-.014	2	-1.047e-2	3	428.739	2	NC	1
59	11	max	0	15	.64	3	.023	1	1.299e-2	2	NC	15	NC	1
60		min	0	1	-.952	2	-.006	10	-9.696e-3	3	393.173	1	NC	1
61	12	max	0	15	.779	3	.079	1	1.198e-2	2	NC	15	NC	2
62		min	0	1	-1.065	1	.001	10	-8.922e-3	3	331.642	1	3187.26	1
63	13	max	0	15	.924	3	.134	1	1.098e-2	2	NC	15	NC	3
64		min	0	1	-1.18	1	.006	15	-8.148e-3	3	287.274	1	1865.668	1
65	14	max	0	15	1.022	3	.167	1	9.982e-3	2	9990.072	15	NC	3
66		min	0	1	-1.246	1	.007	15	-7.373e-3	3	266.58	1	1488	1
67	15	max	0	15	1.039	3	.169	1	8.98e-3	2	NC	15	NC	3
68		min	0	1	-1.234	1	.007	15	-6.599e-3	3	270.247	1	1470.273	1
69	16	max	0	15	.956	3	.139	1	7.979e-3	2	NC	15	NC	3
70		min	0	1	-1.127	1	.006	15	-5.825e-3	3	306.159	1	1790.124	1
71	17	max	0	15	.774	3	.087	1	6.977e-3	2	NC	15	NC	3
72		min	0	1	-.926	1	.004	15	-5.051e-3	3	408.039	1	2891.683	1
73	18	max	0	15	.509	3	.031	1	5.976e-3	2	NC	5	NC	2
74		min	0	1	-.648	1	0	10	-4.276e-3	3	758.144	1	8323.993	1
75	19	max	0	15	.195	3	.006	3	4.974e-3	2	NC	1	NC	1
76		min	0	1	-.328	2	-.003	2	-3.502e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.199	.006	3	2.956e-3	3	NC	1	NC	1
78		min	0	1	-.327	2	-.003	2	-5.165e-3	2	NC	1	NC	1
79	2	max	0	15	.391	3	.031	1	3.614e-3	3	NC	5	NC	2
80		min	0	1	-.718	2	0	10	-6.209e-3	2	628.871	2	8284.498	1
81	3	max	0	15	.558	3	.087	1	4.273e-3	3	NC	15	NC	3
82		min	0	1	-1.051	2	.004	15	-7.252e-3	2	339.934	2	2883.5	1
83	4	max	0	15	.68	3	.139	1	4.931e-3	3	NC	15	NC	3
84		min	0	1	-1.284	2	.006	15	-8.296e-3	2	256.978	2	1786.121	1
85	5	max	0	15	.749	3	.169	1	5.589e-3	3	NC	15	NC	3
86		min	0	1	-1.399	2	.007	15	-9.339e-3	2	229.48	2	1467.231	1
87	6	max	0	15	.765	3	.167	1	6.248e-3	3	NC	15	NC	3
88		min	0	1	-1.395	2	.007	15	-1.038e-2	2	230.352	2	1484.727	1
89	7	max	0	15	.736	3	.134	1	6.906e-3	3	NC	15	NC	3
90		min	0	1	-1.292	2	.006	15	-1.143e-2	2	254.809	2	1860.49	1
91	8	max	0	15	.678	3	.079	1	7.565e-3	3	NC	15	NC	2
92		min	0	1	-1.132	2	.002	10	-1.247e-2	2	305.604	2	3172.423	1
93	9	max	0	15	.617	3	.023	1	8.223e-3	3	NC	15	NC	1
94		min	0	1	-.974	2	-.005	10	-1.351e-2	2	380.101	2	NC	1
95	10	max	0	1	.587	3	.019	3	8.882e-3	3	NC	5	NC	1
96		min	0	1	-.9	2	-.013	2	-1.456e-2	2	429.356	2	NC	1
97	11	max	0	1	.617	3	.023	1	8.223e-3	3	NC	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-.974	2	-.005	10	-1.351e-2	2	380.101	2	NC	1
99		max	0	1	.678	3	.079	1	7.565e-3	3	NC	15	NC	2
100		min	0	15	-1.132	2	.002	10	-1.247e-2	2	305.604	2	3172.423	1
101		max	0	1	.736	3	.134	1	6.906e-3	3	NC	15	NC	3
102		min	0	15	-1.292	2	.006	15	-1.143e-2	2	254.809	2	1860.49	1
103		max	0	1	.765	3	.167	1	6.248e-3	3	NC	15	NC	3
104		min	0	15	-1.395	2	.007	15	-1.038e-2	2	230.352	2	1484.727	1
105		max	0	1	.749	3	.169	1	5.589e-3	3	NC	15	NC	3
106		min	0	15	-1.399	2	.007	15	-9.339e-3	2	229.48	2	1467.231	1
107		max	0	1	.68	3	.139	1	4.931e-3	3	NC	15	NC	3
108		min	0	15	-1.284	2	.006	15	-8.296e-3	2	256.978	2	1786.121	1
109		max	0	1	.558	3	.087	1	4.273e-3	3	NC	15	NC	3
110		min	0	15	-1.051	2	.004	15	-7.252e-3	2	339.934	2	2883.5	1
111		max	0	1	.391	3	.031	1	3.614e-3	3	NC	5	NC	2
112		min	0	15	-.718	2	0	10	-6.209e-3	2	628.871	2	8284.498	1
113		max	0	1	.199	3	.006	3	2.956e-3	3	NC	1	NC	1
114		min	0	15	-.327	2	-.003	2	-5.165e-3	2	NC	1	NC	1
115	M16	max	0	15	.096	1	.005	3	5.245e-3	3	NC	1	NC	1
116		min	-.001	1	-.065	3	-.003	2	-7.367e-3	1	NC	1	NC	1
117		max	0	15	.044	3	.044	1	6.209e-3	3	NC	5	NC	2
118		min	-.001	1	-.185	2	.002	15	-8.434e-3	1	890.075	2	5730.983	1
119		max	0	15	.129	3	.107	1	7.173e-3	3	NC	5	NC	3
120		min	0	1	-.406	2	.005	15	-9.5e-3	1	495.047	2	2329.477	1
121		max	0	15	.175	3	.161	1	8.137e-3	3	NC	5	NC	3
122		min	0	1	-.533	2	.007	15	-1.057e-2	1	394.059	2	1536.166	1
123		max	0	15	.174	3	.19	1	9.1e-3	3	NC	5	NC	3
124		min	0	1	-.55	2	.008	15	-1.163e-2	1	383.627	2	1305.14	1
125		max	0	15	.128	3	.184	1	1.006e-2	3	NC	5	NC	3
126		min	0	1	-.46	2	.008	15	-1.27e-2	1	446.589	2	1348.342	1
127		max	0	15	.046	3	.145	1	1.103e-2	3	NC	5	NC	3
128		min	0	1	-.285	2	.006	15	-1.376e-2	1	653.89	2	1712.202	1
129		max	0	15	.004	4	.085	1	1.199e-2	3	NC	3	NC	3
130		min	0	1	-.069	2	.003	10	-1.483e-2	1	1530.947	2	2939.978	1
131		max	0	15	.145	1	.025	1	1.296e-2	3	NC	4	NC	1
132		min	0	1	-.138	3	-.004	10	-1.59e-2	1	3402.553	3	NC	1
133		max	0	1	.225	1	.016	3	1.392e-2	3	NC	5	NC	1
134		min	0	1	-.176	3	-.012	2	-1.696e-2	1	1907.915	1	NC	1
135		max	0	1	.145	1	.025	1	1.296e-2	3	NC	4	NC	1
136		min	0	15	-.138	3	-.004	10	-1.59e-2	1	3402.553	3	NC	1
137		max	0	1	.004	4	.085	1	1.199e-2	3	NC	3	NC	3
138		min	0	15	-.069	2	.003	10	-1.483e-2	1	1530.947	2	2939.978	1
139		max	0	1	.046	3	.145	1	1.103e-2	3	NC	5	NC	3
140		min	0	15	-.285	2	.006	15	-1.376e-2	1	653.89	2	1712.202	1
141		max	0	1	.128	3	.184	1	1.006e-2	3	NC	5	NC	3
142		min	0	15	-.46	2	.008	15	-1.27e-2	1	446.589	2	1348.342	1
143		max	0	1	.174	3	.19	1	9.1e-3	3	NC	5	NC	3
144		min	0	15	-.55	2	.008	15	-1.163e-2	1	383.627	2	1305.14	1
145		max	0	1	.175	3	.161	1	8.137e-3	3	NC	5	NC	3
146		min	0	15	-.533	2	.007	15	-1.057e-2	1	394.059	2	1536.166	1
147		max	0	1	.129	3	.107	1	7.173e-3	3	NC	5	NC	3
148		min	0	15	-.406	2	.005	15	-9.5e-3	1	495.047	2	2329.477	1
149		max	.001	1	.044	3	.044	1	6.209e-3	3	NC	5	NC	2
150		min	0	15	-.185	2	.002	15	-8.434e-3	1	890.075	2	5730.983	1
151		max	.001	1	.096	1	.005	3	5.245e-3	3	NC	1	NC	1
152		min	0	15	-.065	3	-.003	2	-7.367e-3	1	NC	1	NC	1
153	M2	max	.006	1	.006	2	.009	1	-1.007e-5	15	NC	1	NC	2
154		min	-.007	3	-.01	3	0	15	-2.438e-4	1	NC	1	7110.757	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.006	1	.005	2	.008	1	-9.451e-6	15	NC	1	NC	2
156			min	-.007	3	-.01	3	0	15	-2.287e-4	1	NC	1	7756.839	1
157		3	max	.005	1	.004	2	.007	1	-8.828e-6	15	NC	1	NC	2
158			min	-.006	3	-.009	3	0	15	-2.136e-4	1	NC	1	8527.128	1
159		4	max	.005	1	.003	2	.007	1	-8.204e-6	15	NC	1	NC	2
160			min	-.006	3	-.009	3	0	15	-1.985e-4	1	NC	1	9454.637	1
161		5	max	.005	1	.002	2	.006	1	-7.581e-6	15	NC	1	NC	1
162			min	-.006	3	-.009	3	0	15	-1.834e-4	1	NC	1	NC	1
163		6	max	.004	1	.002	2	.005	1	-6.958e-6	15	NC	1	NC	1
164			min	-.005	3	-.008	3	0	15	-1.683e-4	1	NC	1	NC	1
165		7	max	.004	1	.001	2	.005	1	-6.335e-6	15	NC	1	NC	1
166			min	-.005	3	-.008	3	0	15	-1.532e-4	1	NC	1	NC	1
167		8	max	.004	1	0	2	.004	1	-5.712e-6	15	NC	1	NC	1
168			min	-.004	3	-.007	3	0	15	-1.381e-4	1	NC	1	NC	1
169		9	max	.003	1	0	2	.003	1	-5.088e-6	15	NC	1	NC	1
170			min	-.004	3	-.007	3	0	15	-1.23e-4	1	NC	1	NC	1
171		10	max	.003	1	0	2	.003	1	-4.465e-6	15	NC	1	NC	1
172			min	-.004	3	-.006	3	0	15	-1.079e-4	1	NC	1	NC	1
173		11	max	.003	1	0	2	.002	1	-3.842e-6	15	NC	1	NC	1
174			min	-.003	3	-.006	3	0	15	-9.285e-5	1	NC	1	NC	1
175		12	max	.002	1	0	15	.002	1	-3.219e-6	15	NC	1	NC	1
176			min	-.003	3	-.005	3	0	15	-7.776e-5	1	NC	1	NC	1
177		13	max	.002	1	0	15	.001	1	-2.595e-6	15	NC	1	NC	1
178			min	-.002	3	-.005	3	0	15	-6.267e-5	1	NC	1	NC	1
179		14	max	.002	1	0	15	0	1	-1.972e-6	15	NC	1	NC	1
180			min	-.002	3	-.004	3	0	15	-4.758e-5	1	NC	1	NC	1
181		15	max	.001	1	0	15	0	1	-1.349e-6	15	NC	1	NC	1
182			min	-.002	3	-.003	3	0	15	-3.249e-5	1	NC	1	NC	1
183		16	max	.001	1	0	15	0	1	-7.257e-7	15	NC	1	NC	1
184			min	-.001	3	-.003	3	0	15	-1.74e-5	1	NC	1	NC	1
185		17	max	0	1	0	15	0	1	-1.025e-7	15	NC	1	NC	1
186			min	0	3	-.002	4	0	15	-2.309e-6	1	NC	1	NC	1
187		18	max	0	1	0	15	0	1	1.278e-5	1	NC	1	NC	1
188			min	0	3	-.001	4	0	15	4.173e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.787e-5	1	NC	1	NC	1
190			min	0	1	0	1	0	1	1.144e-6	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-3.79e-7	15	NC	1	NC	1
192			min	0	1	0	1	0	1	-9.224e-6	1	NC	1	NC	1
193		2	max	0	3	0	15	0	1	1.627e-5	1	NC	1	NC	1
194			min	0	2	-.002	4	0	15	6.722e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	1	4.176e-5	1	NC	1	NC	1
196			min	0	2	-.004	4	0	15	1.723e-6	15	NC	1	NC	1
197		4	max	0	3	-.001	15	0	1	6.726e-5	1	NC	1	NC	1
198			min	0	2	-.006	4	0	15	2.774e-6	15	NC	1	NC	1
199		5	max	.001	3	-.002	15	0	1	9.275e-5	1	NC	1	NC	1
200			min	0	2	-.007	4	0	15	3.826e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	1	1.182e-4	1	NC	1	NC	1
202			min	-.001	2	-.009	4	0	15	4.877e-6	15	NC	1	NC	1
203		7	max	.002	3	-.003	15	.001	1	1.437e-4	1	NC	1	NC	1
204			min	-.001	2	-.011	4	0	15	5.928e-6	15	8605.229	4	NC	1
205		8	max	.002	3	-.003	15	.002	1	1.692e-4	1	NC	1	NC	1
206			min	-.002	2	-.012	4	0	15	6.979e-6	15	7725.855	4	NC	1
207		9	max	.003	3	-.003	15	.002	1	1.947e-4	1	NC	2	NC	1
208			min	-.002	2	-.013	4	0	15	8.03e-6	15	7205.926	4	NC	1
209		10	max	.003	3	-.003	15	.002	1	2.202e-4	1	NC	3	NC	1
210			min	-.002	2	-.013	4	0	15	9.081e-6	15	6954.865	4	NC	1
211		11	max	.003	3	-.003	15	.003	1	2.457e-4	1	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.002	2	-.014	4	0	15	1.013e-5	15	6935.686	4	NC	1
213		max	.003	3	-.003	15	.003	1	2.712e-4	1	NC	2	NC	1
214		min	-.003	2	-.013	4	0	15	1.118e-5	15	7150.809	4	NC	1
215		max	.004	3	-.003	15	.004	1	2.967e-4	1	NC	1	NC	1
216		min	-.003	2	-.012	4	0	15	1.223e-5	15	7644.805	4	NC	1
217		max	.004	3	-.003	15	.004	1	3.222e-4	1	NC	1	NC	1
218		min	-.003	2	-.011	4	0	15	1.329e-5	15	8527.5	4	NC	1
219		max	.004	3	-.002	15	.005	1	3.477e-4	1	NC	1	NC	1
220		min	-.003	2	-.01	4	0	15	1.434e-5	15	NC	1	NC	1
221		max	.005	3	-.002	15	.006	1	3.732e-4	1	NC	1	NC	1
222		min	-.004	2	-.008	4	0	15	1.539e-5	15	NC	1	NC	1
223		max	.005	3	-.001	15	.007	1	3.987e-4	1	NC	1	NC	1
224		min	-.004	2	-.006	1	0	15	1.644e-5	15	NC	1	NC	1
225		max	.005	3	0	15	.007	1	4.242e-4	1	NC	1	NC	1
226		min	-.004	2	-.004	1	0	15	1.749e-5	15	NC	1	NC	1
227		max	.006	3	0	15	.008	1	4.496e-4	1	NC	1	NC	1
228		min	-.004	2	-.003	1	0	15	1.854e-5	15	NC	1	NC	1
229	M4	max	.003	1	.004	2	0	15	6.242e-5	1	NC	1	NC	3
230		min	0	3	-.006	3	-.008	1	2.587e-6	15	NC	1	2953.069	1
231		max	.003	1	.004	2	0	15	6.242e-5	1	NC	1	NC	3
232		min	0	3	-.006	3	-.008	1	2.587e-6	15	NC	1	3210.767	1
233		max	.002	1	.004	2	0	15	6.242e-5	1	NC	1	NC	3
234		min	0	3	-.005	3	-.007	1	2.587e-6	15	NC	1	3517.497	1
235		max	.002	1	.003	2	0	15	6.242e-5	1	NC	1	NC	3
236		min	0	3	-.005	3	-.006	1	2.587e-6	15	NC	1	3885.988	1
237		max	.002	1	.003	2	0	15	6.242e-5	1	NC	1	NC	2
238		min	0	3	-.005	3	-.006	1	2.587e-6	15	NC	1	4333.546	1
239		max	.002	1	.003	2	0	15	6.242e-5	1	NC	1	NC	2
240		min	0	3	-.004	3	-.005	1	2.587e-6	15	NC	1	4884.163	1
241		max	.002	1	.003	2	0	15	6.242e-5	1	NC	1	NC	2
242		min	0	3	-.004	3	-.004	1	2.587e-6	15	NC	1	5571.903	1
243		max	.002	1	.002	2	0	15	6.242e-5	1	NC	1	NC	2
244		min	0	3	-.004	3	-.004	1	2.587e-6	15	NC	1	6446.44	1
245		max	.002	1	.002	2	0	15	6.242e-5	1	NC	1	NC	2
246		min	0	3	-.003	3	-.003	1	2.587e-6	15	NC	1	7582.557	1
247		max	.001	1	.002	2	0	15	6.242e-5	1	NC	1	NC	2
248		min	0	3	-.003	3	-.003	1	2.587e-6	15	NC	1	9097.153	1
249		max	.001	1	.002	2	0	15	6.242e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.002	1	2.587e-6	15	NC	1	NC	1
251		max	.001	1	.002	2	0	15	6.242e-5	1	NC	1	NC	1
252		min	0	3	-.002	3	-.002	1	2.587e-6	15	NC	1	NC	1
253		max	0	1	.001	2	0	15	6.242e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	-.001	1	2.587e-6	15	NC	1	NC	1
255		max	0	1	.001	2	0	15	6.242e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	2.587e-6	15	NC	1	NC	1
257		max	0	1	0	2	0	15	6.242e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	2.587e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	6.242e-5	1	NC	1	NC	1
260		min	0	3	0	3	0	1	2.587e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	6.242e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	2.587e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	6.242e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	2.587e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	6.242e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	2.587e-6	15	NC	1	NC	1
267	M6	max	.02	1	.022	2	0	1	0	1	NC	4	NC	1
268		min	-.023	3	-.032	3	0	1	0	1	1977.296	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.018	1	.02	2	0	1	0	1	NC	4	NC	1
270		min	-.022	3	-.03	3	0	1	0	1	2093.522	3	NC	1
271	3	max	.017	1	.019	2	0	1	0	1	NC	4	NC	1
272		min	-.021	3	-.028	3	0	1	0	1	2224.332	3	NC	1
273	4	max	.016	1	.017	2	0	1	0	1	NC	4	NC	1
274		min	-.019	3	-.026	3	0	1	0	1	2372.711	3	NC	1
275	5	max	.015	1	.015	2	0	1	0	1	NC	4	NC	1
276		min	-.018	3	-.025	3	0	1	0	1	2542.499	3	NC	1
277	6	max	.014	1	.013	2	0	1	0	1	NC	1	NC	1
278		min	-.017	3	-.023	3	0	1	0	1	2738.718	3	NC	1
279	7	max	.013	1	.011	2	0	1	0	1	NC	1	NC	1
280		min	-.016	3	-.021	3	0	1	0	1	2968.063	3	NC	1
281	8	max	.012	1	.01	2	0	1	0	1	NC	1	NC	1
282		min	-.014	3	-.019	3	0	1	0	1	3239.666	3	NC	1
283	9	max	.011	1	.008	2	0	1	0	1	NC	1	NC	1
284		min	-.013	3	-.018	3	0	1	0	1	3566.31	3	NC	1
285	10	max	.01	1	.007	2	0	1	0	1	NC	1	NC	1
286		min	-.012	3	-.016	3	0	1	0	1	3966.464	3	NC	1
287	11	max	.009	1	.005	2	0	1	0	1	NC	1	NC	1
288		min	-.01	3	-.014	3	0	1	0	1	4467.832	3	NC	1
289	12	max	.008	1	.004	2	0	1	0	1	NC	1	NC	1
290		min	-.009	3	-.012	3	0	1	0	1	5113.952	3	NC	1
291	13	max	.007	1	.003	2	0	1	0	1	NC	1	NC	1
292		min	-.008	3	-.01	3	0	1	0	1	5977.395	3	NC	1
293	14	max	.005	1	.002	2	0	1	0	1	NC	1	NC	1
294		min	-.006	3	-.009	3	0	1	0	1	7188.794	3	NC	1
295	15	max	.004	1	.001	2	0	1	0	1	NC	1	NC	1
296		min	-.005	3	-.007	3	0	1	0	1	9009.426	3	NC	1
297	16	max	.003	1	0	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	1	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.003	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	1	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	0	3	0	2	0	1	0	1	NC	1	NC	1
308		min	0	2	-.002	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	0	15	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.005	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
313	5	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.009	3	0	1	0	1	NC	1	NC	1
315	6	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.011	3	0	1	0	1	9516.726	3	NC	1
317	7	max	.006	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.006	2	-.012	3	0	1	0	1	8467.538	3	NC	1
319	8	max	.007	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.007	2	-.013	3	0	1	0	1	7841.647	3	NC	1
321	9	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.008	2	-.014	3	0	1	0	1	7356.536	4	NC	1
323	10	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.009	2	-.014	3	0	1	0	1	7091.678	4	NC	1
325	11	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.01	2	-.014	3	0	1	0	1	7064.958	4	NC	1
327		12	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.011	2	-.014	3	0	1	0	1	7277.876	4	NC	1
329		13	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.011	2	-.013	3	0	1	0	1	7775.079	4	NC	1
331		14	max	.013	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.012	2	-.012	3	0	1	0	1	8667.655	4	NC	1
333		15	max	.014	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.013	2	-.011	3	0	1	0	1	NC	1	NC	1
335		16	max	.015	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.014	2	-.009	3	0	1	0	1	NC	1	NC	1
337		17	max	.016	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.015	2	-.008	3	0	1	0	1	NC	1	NC	1
339		18	max	.017	3	0	15	0	1	0	1	NC	1	NC	1
340			min	-.016	2	-.006	1	0	1	0	1	NC	1	NC	1
341		19	max	.018	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.017	2	-.005	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.016	2	0	1	0	1	NC	1	NC	1
344			min	-.001	3	-.018	3	0	1	0	1	NC	1	NC	1
345		2	max	.007	1	.015	2	0	1	0	1	NC	1	NC	1
346			min	-.001	3	-.017	3	0	1	0	1	NC	1	NC	1
347		3	max	.006	1	.014	2	0	1	0	1	NC	1	NC	1
348			min	0	3	-.016	3	0	1	0	1	NC	1	NC	1
349		4	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
350			min	0	3	-.015	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	1	.013	2	0	1	0	1	NC	1	NC	1
352			min	0	3	-.014	3	0	1	0	1	NC	1	NC	1
353		6	max	.005	1	.012	2	0	1	0	1	NC	1	NC	1
354			min	0	3	-.013	3	0	1	0	1	NC	1	NC	1
355		7	max	.005	1	.011	2	0	1	0	1	NC	1	NC	1
356			min	0	3	-.012	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	1	.01	2	0	1	0	1	NC	1	NC	1
358			min	0	3	-.011	3	0	1	0	1	NC	1	NC	1
359		9	max	.004	1	.009	2	0	1	0	1	NC	1	NC	1
360			min	0	3	-.01	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
362			min	0	3	-.009	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.007	2	0	1	0	1	NC	1	NC	1
364			min	0	3	-.008	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.007	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	1	.006	2	0	15	2.438e-4	1	NC	1	NC	2
382			min	-.007	3	-.01	3	-.009	1	1.007e-5	15	NC	1	7110.757	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383		2	max	.006	1	.005	2	0	15	2.287e-4	1	NC	1	NC	2
384			min	-.007	3	-.01	3	-.008	1	9.451e-6	15	NC	1	7756.839	1
385		3	max	.005	1	.004	2	0	15	2.136e-4	1	NC	1	NC	2
386			min	-.006	3	-.009	3	-.007	1	8.828e-6	15	NC	1	8527.128	1
387		4	max	.005	1	.003	2	0	15	1.985e-4	1	NC	1	NC	2
388			min	-.006	3	-.009	3	-.007	1	8.204e-6	15	NC	1	9454.637	1
389		5	max	.005	1	.002	2	0	15	1.834e-4	1	NC	1	NC	1
390			min	-.006	3	-.009	3	-.006	1	7.581e-6	15	NC	1	NC	1
391		6	max	.004	1	.002	2	0	15	1.683e-4	1	NC	1	NC	1
392			min	-.005	3	-.008	3	-.005	1	6.958e-6	15	NC	1	NC	1
393		7	max	.004	1	.001	2	0	15	1.532e-4	1	NC	1	NC	1
394			min	-.005	3	-.008	3	-.005	1	6.335e-6	15	NC	1	NC	1
395		8	max	.004	1	0	2	0	15	1.381e-4	1	NC	1	NC	1
396			min	-.004	3	-.007	3	-.004	1	5.712e-6	15	NC	1	NC	1
397		9	max	.003	1	0	2	0	15	1.23e-4	1	NC	1	NC	1
398			min	-.004	3	-.007	3	-.003	1	5.088e-6	15	NC	1	NC	1
399		10	max	.003	1	0	2	0	15	1.079e-4	1	NC	1	NC	1
400			min	-.004	3	-.006	3	-.003	1	4.465e-6	15	NC	1	NC	1
401		11	max	.003	1	0	2	0	15	9.285e-5	1	NC	1	NC	1
402			min	-.003	3	-.006	3	-.002	1	3.842e-6	15	NC	1	NC	1
403		12	max	.002	1	0	15	0	15	7.776e-5	1	NC	1	NC	1
404			min	-.003	3	-.005	3	-.002	1	3.219e-6	15	NC	1	NC	1
405		13	max	.002	1	0	15	0	15	6.267e-5	1	NC	1	NC	1
406			min	-.002	3	-.005	3	-.001	1	2.595e-6	15	NC	1	NC	1
407		14	max	.002	1	0	15	0	15	4.758e-5	1	NC	1	NC	1
408			min	-.002	3	-.004	3	0	1	1.972e-6	15	NC	1	NC	1
409		15	max	.001	1	0	15	0	15	3.249e-5	1	NC	1	NC	1
410			min	-.002	3	-.003	3	0	1	1.349e-6	15	NC	1	NC	1
411		16	max	.001	1	0	15	0	15	1.74e-5	1	NC	1	NC	1
412			min	-.001	3	-.003	3	0	1	7.257e-7	15	NC	1	NC	1
413		17	max	0	1	0	15	0	15	2.309e-6	1	NC	1	NC	1
414			min	0	3	-.002	4	0	1	1.025e-7	15	NC	1	NC	1
415		18	max	0	1	0	15	0	15	-4.173e-7	12	NC	1	NC	1
416			min	0	3	-.001	4	0	1	-1.278e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	-1.144e-6	15	NC	1	NC	1
418			min	0	1	0	1	0	1	-2.787e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	9.224e-6	1	NC	1	NC	1
420			min	0	1	0	1	0	1	3.79e-7	15	NC	1	NC	1
421		2	max	0	3	0	15	0	15	-6.722e-7	15	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-1.627e-5	1	NC	1	NC	1
423		3	max	0	3	0	15	0	15	-1.723e-6	15	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-4.176e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	0	15	-2.774e-6	15	NC	1	NC	1
426			min	0	2	-.006	4	0	1	-6.726e-5	1	NC	1	NC	1
427		5	max	.001	3	-.002	15	0	15	-3.826e-6	15	NC	1	NC	1
428			min	0	2	-.007	4	0	1	-9.275e-5	1	NC	1	NC	1
429		6	max	.002	3	-.002	15	0	15	-4.877e-6	15	NC	1	NC	1
430			min	-.001	2	-.009	4	0	1	-1.182e-4	1	NC	1	NC	1
431		7	max	.002	3	-.003	15	0	15	-5.928e-6	15	NC	1	NC	1
432			min	-.001	2	-.011	4	-.001	1	-1.437e-4	1	8605.229	4	NC	1
433		8	max	.002	3	-.003	15	0	15	-6.979e-6	15	NC	1	NC	1
434			min	-.002	2	-.012	4	-.002	1	-1.692e-4	1	7725.855	4	NC	1
435		9	max	.003	3	-.003	15	0	15	-8.03e-6	15	NC	2	NC	1
436			min	-.002	2	-.013	4	-.002	1	-1.947e-4	1	7205.926	4	NC	1
437		10	max	.003	3	-.003	15	0	15	-9.081e-6	15	NC	3	NC	1
438			min	-.002	2	-.013	4	-.002	1	-2.202e-4	1	6954.865	4	NC	1
439		11	max	.003	3	-.003	15	0	15	-1.013e-5	15	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.002	2	-.014	4	-.003	1	-2.457e-4	1	6935.686	4	NC	1
441		max	.003	3	-.003	15	0	15	-1.118e-5	15	NC	2	NC	1
442		min	-.003	2	-.013	4	-.003	1	-2.712e-4	1	7150.809	4	NC	1
443		max	.004	3	-.003	15	0	15	-1.223e-5	15	NC	1	NC	1
444		min	-.003	2	-.012	4	-.004	1	-2.967e-4	1	7644.805	4	NC	1
445		max	.004	3	-.003	15	0	15	-1.329e-5	15	NC	1	NC	1
446		min	-.003	2	-.011	4	-.004	1	-3.222e-4	1	8527.5	4	NC	1
447		max	.004	3	-.002	15	0	15	-1.434e-5	15	NC	1	NC	1
448		min	-.003	2	-.01	4	-.005	1	-3.477e-4	1	NC	1	NC	1
449		max	.005	3	-.002	15	0	15	-1.539e-5	15	NC	1	NC	1
450		min	-.004	2	-.008	4	-.006	1	-3.732e-4	1	NC	1	NC	1
451		max	.005	3	-.001	15	0	15	-1.644e-5	15	NC	1	NC	1
452		min	-.004	2	-.006	1	-.007	1	-3.987e-4	1	NC	1	NC	1
453		max	.005	3	0	15	0	15	-1.749e-5	15	NC	1	NC	1
454		min	-.004	2	-.004	1	-.007	1	-4.242e-4	1	NC	1	NC	1
455		max	.006	3	0	15	0	15	-1.854e-5	15	NC	1	NC	1
456		min	-.004	2	-.003	1	-.008	1	-4.496e-4	1	NC	1	NC	1
457	M12	max	.003	1	.004	2	.008	1	-2.587e-6	15	NC	1	NC	3
458		min	0	3	-.006	3	0	15	-6.242e-5	1	NC	1	2953.069	1
459		max	.003	1	.004	2	.008	1	-2.587e-6	15	NC	1	NC	3
460		min	0	3	-.006	3	0	15	-6.242e-5	1	NC	1	3210.767	1
461		max	.002	1	.004	2	.007	1	-2.587e-6	15	NC	1	NC	3
462		min	0	3	-.005	3	0	15	-6.242e-5	1	NC	1	3517.497	1
463		max	.002	1	.003	2	.006	1	-2.587e-6	15	NC	1	NC	3
464		min	0	3	-.005	3	0	15	-6.242e-5	1	NC	1	3885.988	1
465		max	.002	1	.003	2	.006	1	-2.587e-6	15	NC	1	NC	2
466		min	0	3	-.005	3	0	15	-6.242e-5	1	NC	1	4333.546	1
467		max	.002	1	.003	2	.005	1	-2.587e-6	15	NC	1	NC	2
468		min	0	3	-.004	3	0	15	-6.242e-5	1	NC	1	4884.163	1
469		max	.002	1	.003	2	.004	1	-2.587e-6	15	NC	1	NC	2
470		min	0	3	-.004	3	0	15	-6.242e-5	1	NC	1	5571.903	1
471		max	.002	1	.002	2	.004	1	-2.587e-6	15	NC	1	NC	2
472		min	0	3	-.004	3	0	15	-6.242e-5	1	NC	1	6446.44	1
473		max	.002	1	.002	2	.003	1	-2.587e-6	15	NC	1	NC	2
474		min	0	3	-.003	3	0	15	-6.242e-5	1	NC	1	7582.557	1
475		max	.001	1	.002	2	.003	1	-2.587e-6	15	NC	1	NC	2
476		min	0	3	-.003	3	0	15	-6.242e-5	1	NC	1	9097.153	1
477		max	.001	1	.002	2	.002	1	-2.587e-6	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-6.242e-5	1	NC	1	NC	1
479		max	.001	1	.002	2	.002	1	-2.587e-6	15	NC	1	NC	1
480		min	0	3	-.002	3	0	15	-6.242e-5	1	NC	1	NC	1
481		max	0	1	.001	2	.001	1	-2.587e-6	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-6.242e-5	1	NC	1	NC	1
483		max	0	1	.001	2	0	1	-2.587e-6	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-6.242e-5	1	NC	1	NC	1
485		max	0	1	0	2	0	1	-2.587e-6	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-6.242e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-2.587e-6	15	NC	1	NC	1
488		min	0	3	0	3	0	15	-6.242e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-2.587e-6	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-6.242e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-2.587e-6	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-6.242e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-2.587e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-6.242e-5	1	NC	1	NC	1
495	M1	max	.007	3	.102	2	.001	1	1.581e-2	1	NC	1	NC	1
496		min	-.003	2	-.012	3	0	15	-2.536e-2	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.007	3	.048	2	0	15	7.672e-3	1	NC	3	NC	1
498			min	-.003	2	-.004	3	-.006	1	-1.255e-2	3	2170.338	2	NC	1
499		3	max	.007	3	.011	3	0	15	2.252e-5	10	NC	5	NC	1
500			min	-.003	2	-.009	2	-.009	1	-1.752e-4	1	1044.07	2	NC	1
501		4	max	.007	3	.038	3	0	15	4.376e-3	1	NC	5	NC	1
502			min	-.003	2	-.074	2	-.008	1	-4.491e-3	3	657.354	2	NC	1
503		5	max	.007	3	.074	3	0	15	8.927e-3	1	NC	15	NC	1
504			min	-.003	2	-.141	2	-.006	1	-8.86e-3	3	473.374	2	NC	1
505		6	max	.007	3	.114	3	0	15	1.348e-2	1	NC	15	NC	1
506			min	-.003	2	-.208	2	-.002	1	-1.323e-2	3	372.196	2	NC	1
507		7	max	.007	3	.153	3	0	1	1.803e-2	1	NC	15	NC	1
508			min	-.003	2	-.266	2	0	12	-1.76e-2	3	312.559	2	NC	1
509		8	max	.006	3	.185	3	.001	1	2.258e-2	1	9040.965	15	NC	1
510			min	-.003	2	-.313	2	0	15	-2.197e-2	3	277.322	2	NC	1
511		9	max	.006	3	.206	3	0	15	2.498e-2	1	8448.495	15	NC	1
512			min	-.003	2	-.343	2	0	1	-2.197e-2	3	258.994	2	NC	1
513		10	max	.006	3	.213	3	0	1	2.634e-2	2	8268.069	15	NC	1
514			min	-.003	2	-.353	2	0	12	-1.907e-2	3	253.602	2	NC	1
515		11	max	.006	3	.208	3	0	1	2.829e-2	2	8448.226	15	NC	1
516			min	-.003	2	-.343	2	0	15	-1.617e-2	3	259.787	2	NC	1
517		12	max	.006	3	.191	3	0	15	2.73e-2	2	9040.402	15	NC	1
518			min	-.003	2	-.312	2	-.001	1	-1.336e-2	3	279.746	2	NC	1
519		13	max	.006	3	.162	3	0	15	2.19e-2	2	NC	15	NC	1
520			min	-.003	2	-.263	2	0	1	-1.069e-2	3	318.481	2	NC	1
521		14	max	.006	3	.126	3	.002	1	1.651e-2	2	NC	15	NC	1
522			min	-.003	2	-.202	2	0	15	-8.027e-3	3	384.692	1	NC	1
523		15	max	.005	3	.086	3	.005	1	1.111e-2	2	NC	15	NC	1
524			min	-.003	2	-.135	2	0	15	-5.36e-3	3	496.468	1	NC	1
525		16	max	.005	3	.044	3	.008	1	5.708e-3	2	NC	5	NC	1
526			min	-.003	2	-.067	2	0	15	-2.693e-3	3	702.862	1	NC	1
527		17	max	.005	3	.004	3	.008	1	5.799e-4	1	NC	5	NC	1
528			min	-.003	2	-.005	2	0	15	-2.662e-5	3	1142.753	1	NC	1
529		18	max	.005	3	.049	1	.006	1	1.049e-2	2	NC	4	NC	1
530			min	-.003	2	-.032	3	0	15	-4.11e-3	3	2416.1	1	NC	1
531		19	max	.005	3	.096	1	0	15	2.103e-2	2	NC	1	NC	1
532			min	-.003	2	-.065	3	-.001	1	-8.35e-3	3	NC	1	NC	1
533	M5	1	max	.022	3	.242	2	0	1	0	1	NC	1	NC	1
534			min	-.015	2	-.01	3	0	1	0	1	NC	1	NC	1
535		2	max	.022	3	.114	2	0	1	0	1	NC	5	NC	1
536			min	-.015	2	0	3	0	1	0	1	906.715	2	NC	1
537		3	max	.022	3	.034	3	0	1	0	1	NC	5	NC	1
538			min	-.015	2	-.029	2	0	1	0	1	427.225	2	NC	1
539		4	max	.022	3	.109	3	0	1	0	1	9708.645	15	NC	1
540			min	-.015	2	-.199	2	0	1	0	1	261.994	2	NC	1
541		5	max	.021	3	.214	3	0	1	0	1	6795.98	15	NC	1
542			min	-.015	2	-.382	2	0	1	0	1	184.685	2	NC	1
543		6	max	.021	3	.332	3	0	1	0	1	5233.298	15	NC	1
544			min	-.014	2	-.565	2	0	1	0	1	142.916	2	NC	1
545		7	max	.021	3	.448	3	0	1	0	1	4330.639	15	NC	1
546			min	-.014	2	-.729	2	0	1	0	1	118.653	2	NC	1
547		8	max	.02	3	.545	3	0	1	0	1	3805.698	15	NC	1
548			min	-.014	2	-.861	2	0	1	0	1	104.486	2	NC	1
549		9	max	.02	3	.607	3	0	1	0	1	3536.461	15	NC	1
550			min	-.013	2	-.945	2	0	1	0	1	97.2	2	NC	1
551		10	max	.019	3	.63	3	0	1	0	1	3455.343	15	NC	1
552			min	-.013	2	-.973	2	0	1	0	1	95.061	2	NC	1
553		11	max	.019	3	.614	3	0	1	0	1	3536.556	15	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.013	2	-.945	2	0	1	0	1	97.508	2	NC	1
555		12	max	.018	3	.561	3	0	1	0	1	3805.924	15	NC	1
556			min	-.013	2	-.858	2	0	1	0	1	105.49	2	NC	1
557		13	max	.018	3	.476	3	0	1	0	1	4331.106	15	NC	1
558			min	-.013	2	-.719	2	0	1	0	1	121.114	1	NC	1
559		14	max	.018	3	.368	3	0	1	0	1	5234.221	15	NC	1
560			min	-.012	2	-.547	2	0	1	0	1	147.812	1	NC	1
561		15	max	.017	3	.248	3	0	1	0	1	6797.819	15	NC	1
562			min	-.012	2	-.36	2	0	1	0	1	194.617	1	NC	1
563		16	max	.017	3	.126	3	0	1	0	1	9712.508	15	NC	1
564			min	-.012	2	-.177	2	0	1	0	1	283.313	1	NC	1
565		17	max	.016	3	.011	3	0	1	0	1	NC	5	NC	1
566			min	-.012	2	-.016	2	0	1	0	1	477.543	1	NC	1
567		18	max	.016	3	.115	1	0	1	0	1	NC	5	NC	1
568			min	-.012	2	-.087	3	0	1	0	1	1039.409	1	NC	1
569		19	max	.016	3	.225	1	0	1	0	1	NC	1	NC	1
570			min	-.012	2	-.176	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.007	3	.102	2	0	15	2.536e-2	3	NC	1	NC	1
572			min	-.003	2	-.012	3	-.001	1	-1.581e-2	1	NC	1	NC	1
573		2	max	.007	3	.048	2	.006	1	1.255e-2	3	NC	3	NC	1
574			min	-.003	2	-.004	3	0	15	-7.672e-3	1	2170.338	2	NC	1
575		3	max	.007	3	.011	3	.009	1	1.752e-4	1	NC	5	NC	1
576			min	-.003	2	-.009	2	0	15	-2.252e-5	10	1044.07	2	NC	1
577		4	max	.007	3	.038	3	.008	1	4.491e-3	3	NC	5	NC	1
578			min	-.003	2	-.074	2	0	15	-4.376e-3	1	657.354	2	NC	1
579		5	max	.007	3	.074	3	.006	1	8.86e-3	3	NC	15	NC	1
580			min	-.003	2	-.141	2	0	15	-8.927e-3	1	473.374	2	NC	1
581		6	max	.007	3	.114	3	.002	1	1.323e-2	3	NC	15	NC	1
582			min	-.003	2	-.208	2	0	15	-1.348e-2	1	372.196	2	NC	1
583		7	max	.007	3	.153	3	0	12	1.76e-2	3	NC	15	NC	1
584			min	-.003	2	-.266	2	0	1	-1.803e-2	1	312.559	2	NC	1
585		8	max	.006	3	.185	3	0	15	2.197e-2	3	9040.965	15	NC	1
586			min	-.003	2	-.313	2	-.001	1	-2.258e-2	1	277.322	2	NC	1
587		9	max	.006	3	.206	3	0	1	2.197e-2	3	8448.495	15	NC	1
588			min	-.003	2	-.343	2	0	15	-2.498e-2	1	258.994	2	NC	1
589		10	max	.006	3	.213	3	0	12	1.907e-2	3	8268.069	15	NC	1
590			min	-.003	2	-.353	2	0	1	-2.634e-2	2	253.602	2	NC	1
591		11	max	.006	3	.208	3	0	15	1.617e-2	3	8448.226	15	NC	1
592			min	-.003	2	-.343	2	0	1	-2.829e-2	2	259.787	2	NC	1
593		12	max	.006	3	.191	3	.001	1	1.336e-2	3	9040.402	15	NC	1
594			min	-.003	2	-.312	2	0	15	-2.73e-2	2	279.746	2	NC	1
595		13	max	.006	3	.162	3	0	1	1.069e-2	3	NC	15	NC	1
596			min	-.003	2	-.263	2	0	15	-2.19e-2	2	318.481	2	NC	1
597		14	max	.006	3	.126	3	0	15	8.027e-3	3	NC	15	NC	1
598			min	-.003	2	-.202	2	-.002	1	-1.651e-2	2	384.692	1	NC	1
599		15	max	.005	3	.086	3	0	15	5.36e-3	3	NC	15	NC	1
600			min	-.003	2	-.135	2	-.005	1	-1.111e-2	2	496.468	1	NC	1
601		16	max	.005	3	.044	3	0	15	2.693e-3	3	NC	5	NC	1
602			min	-.003	2	-.067	2	-.008	1	-5.708e-3	2	702.862	1	NC	1
603		17	max	.005	3	.004	3	0	15	2.662e-5	3	NC	5	NC	1
604			min	-.003	2	-.005	2	-.008	1	-5.799e-4	1	1142.753	1	NC	1
605		18	max	.005	3	.049	1	0	15	4.11e-3	3	NC	4	NC	1
606			min	-.003	2	-.032	3	-.006	1	-1.049e-2	2	2416.1	1	NC	1
607		19	max	.005	3	.096	1	.001	1	8.35e-3	3	NC	1	NC	1
608			min	-.003	2	-.065	3	0	15	-2.103e-2	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

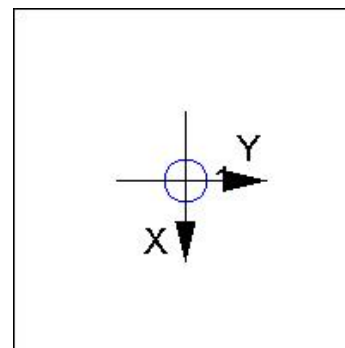
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 1723
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
Adhesive	1723	5365	0.32	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	593	3156	0.19	Pass (Governs)	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

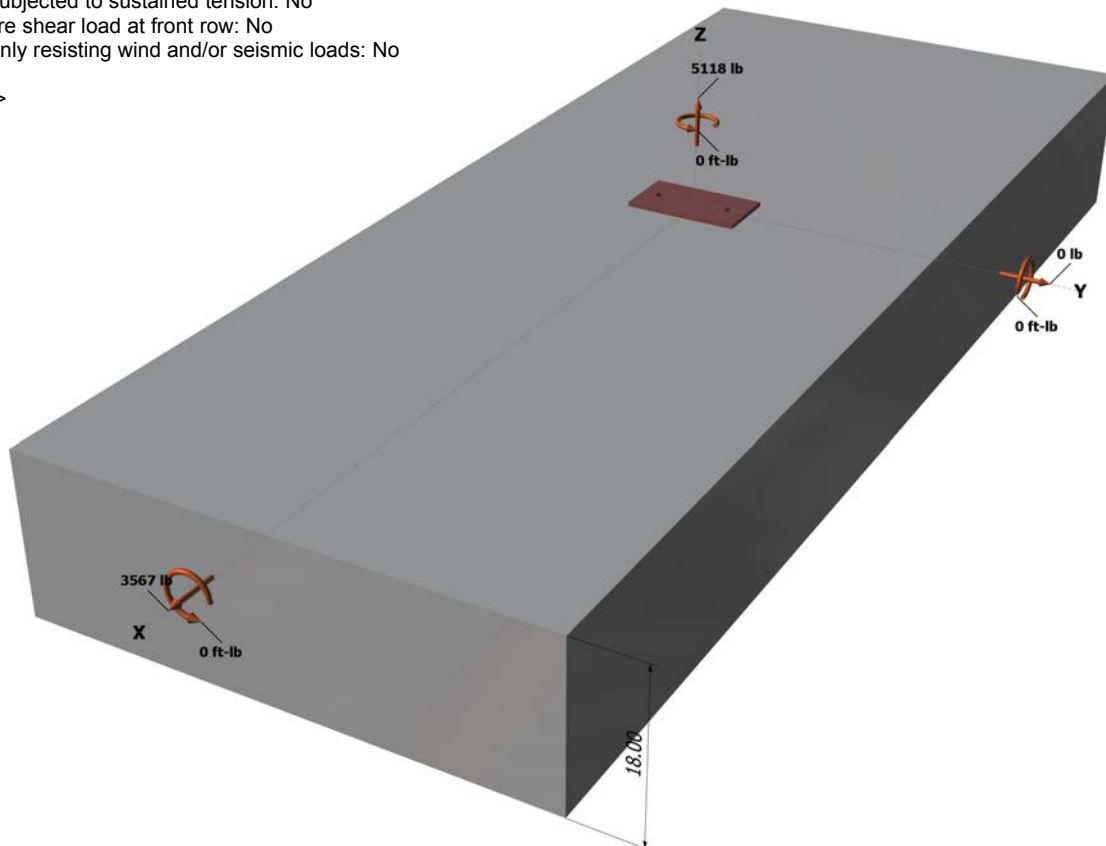
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

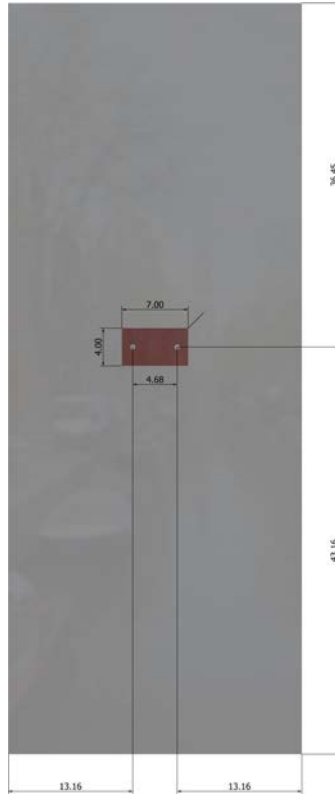
Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

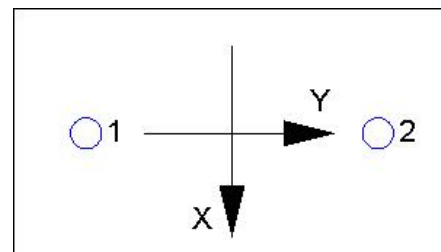
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 5118
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbgx} (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
Adhesive	5118	8093	0.63	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	1784	3156	0.57	Pass (Governs)	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
------------	------	------	---------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.