

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	110 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 19.00 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

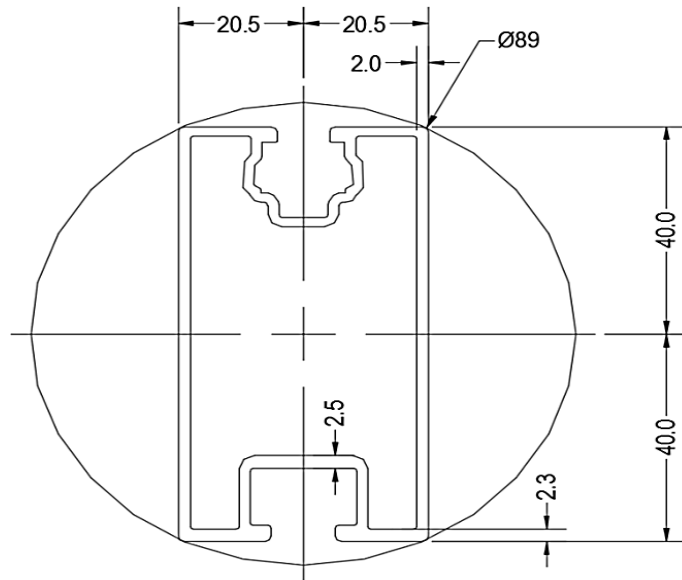
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>ProfiPlusXT</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	28.61 ksi
$\Phi F_{ty}$ WEAK-AXIS =	22.71 ksi
$S_y$ =	0.75 in <sup>3</sup>
$S_x$ =	0.44 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	1.20 in <sup>4</sup>
$I_x$ =	0.36 in <sup>4</sup>
$A$ =	0.96 in <sup>2</sup>
$g$ =	1.15 lbs/ft
$M_y$ =	1.344 k-ft
$M_z$ =	0.193 k-ft
$M_{y \text{ allowable}}$ =	1.778 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	<b>99%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

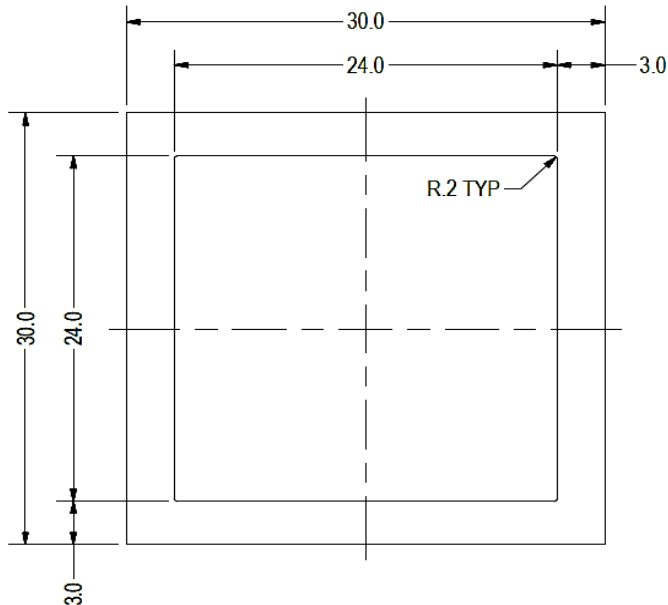
Girder Type =	<b>Flex Profi</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.78 in
$\Phi F_{ty}$ AXIAL =	14.29 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.93 ksi
$\Phi F_{ty}$ WEAK-AXIS =	13.46 ksi
$S_y$ =	0.59 in <sup>3</sup>
$S_x$ =	0.46 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.88 in <sup>4</sup>
$I_x$ =	0.52 in <sup>4</sup>
$A$ =	0.89 in <sup>2</sup>
$g$ =	1.07 lbs/ft
$M_y$ =	0.655 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.201 k
$M_{y \text{ allowable}}$ =	1.469 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	<b>46%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.059 k-ft
$P_n$ =	0.337 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>17%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.151 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>4%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.433 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>18%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.008 k-ft
$P_n$ =	0.271 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>20%</b>



A cross brace kit is required every 10 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

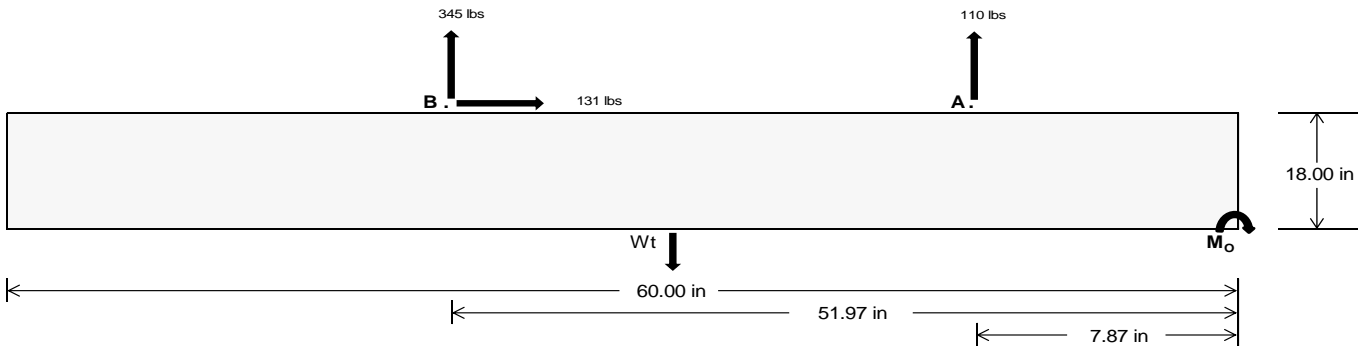
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>486.43</b>	<b>1503.16</b>	k
Compressive Load =	<b>2308.74</b>	<b>1694.29</b>	k
Lateral Load =	<b>47.68</b>	<b>566.11</b>	k
Moment (Weak Axis) =	<b>0.08</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 21156.3$  in-lbs  
Resisting Force Required = 705.21 lbs  
S.F. = 1.67  
Weight Required = 1175.35 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 130.53 lbs  
Friction = 0.4  
Weight Required = 326.33 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 130.53 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

	Ballast Width			
	21 in	22 in	23 in	24 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	885 lbs	885 lbs	885 lbs	885 lbs	611 lbs	611 lbs	611 lbs	611 lbs	1062 lbs	1062 lbs	1062 lbs	1062 lbs	-220 lbs	-220 lbs	-220 lbs	-220 lbs
$F_B$	652 lbs	652 lbs	652 lbs	652 lbs	447 lbs	447 lbs	447 lbs	447 lbs	778 lbs	778 lbs	778 lbs	778 lbs	-690 lbs	-690 lbs	-690 lbs	-690 lbs
$F_V$	62 lbs	62 lbs	62 lbs	62 lbs	234 lbs	234 lbs	234 lbs	234 lbs	218 lbs	218 lbs	218 lbs	218 lbs	-261 lbs	-261 lbs	-261 lbs	-261 lbs
$P_{total}$	3440 lbs	3531 lbs	3622 lbs	3712 lbs	2961 lbs	3052 lbs	3142 lbs	3233 lbs	3743 lbs	3834 lbs	3925 lbs	4015 lbs	231 lbs	286 lbs	340 lbs	395 lbs
$M$	532 lbs-ft	532 lbs-ft	532 lbs-ft	532 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	861 lbs-ft	861 lbs-ft	861 lbs-ft	861 lbs-ft	467 lbs-ft	467 lbs-ft	467 lbs-ft	467 lbs-ft
$e$	0.15 ft	0.15 ft	0.15 ft	0.14 ft	0.22 ft	0.22 ft	0.21 ft	0.20 ft	0.23 ft	0.22 ft	0.22 ft	0.21 ft	2.02 ft	1.63 ft	1.37 ft	1.18 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	320.2 psf	315.5 psf	311.2 psf	307.3 psf	248.0 psf	246.6 psf	245.4 psf	244.2 psf	309.8 psf	305.6 psf	301.8 psf	298.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	466.2 psf	454.9 psf	444.6 psf	435.1 psf	428.8 psf	419.2 psf	410.4 psf	402.4 psf	545.8 psf	530.9 psf	517.3 psf	504.8 psf	182.4 psf	119.9 psf	104.9 psf	99.9 psf

Maximum Bearing Pressure = 546 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

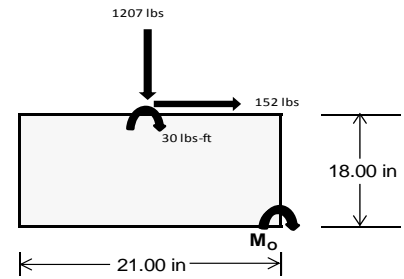
### Overturning Check

$M_o = 798.4 \text{ ft-lbs}$   
 Resisting Force Required = 912.44 lbs  
 S.F. = 1.67  
 Weight Required = 1520.74 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	140 lbs	214 lbs	91 lbs	434 lbs	1207 lbs	395 lbs	74 lbs	28 lbs	28 lbs
$F_v$	25 lbs	200 lbs	26 lbs	17 lbs	152 lbs	20 lbs	25 lbs	200 lbs	26 lbs
$P_{total}$	2496 lbs	2570 lbs	2447 lbs	2677 lbs	3450 lbs	2638 lbs	763 lbs	717 lbs	717 lbs
$M$	72 lbs-ft	340 lbs-ft	78 lbs-ft	48 lbs-ft	257 lbs-ft	61 lbs-ft	74 lbs-ft	340 lbs-ft	78 lbs-ft
$e$	0.03 ft	0.13 ft	0.03 ft	0.02 ft	0.07 ft	0.02 ft	0.10 ft	0.47 ft	0.11 ft
$L/6$	0.29 ft	1.49 ft	1.69 ft	1.71 ft	1.60 ft	1.70 ft	1.56 ft	0.80 ft	1.53 ft
$f_{min}$	256.8 sqft	160.3 sqft	249.2 sqft	287.1 sqft	293.3 sqft	277.7 sqft	58.2 sqft	-51.3 sqft	51.6 sqft
$f_{max}$	313.6 psf	427.1 psf	310.1 psf	324.7 psf	495.1 psf	325.3 psf	116.1 psf	215.2 psf	112.4 psf



Maximum Bearing Pressure = 495 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



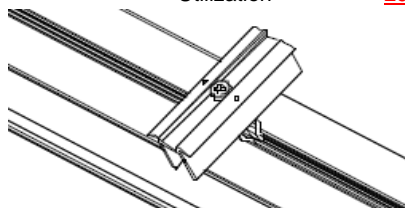
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

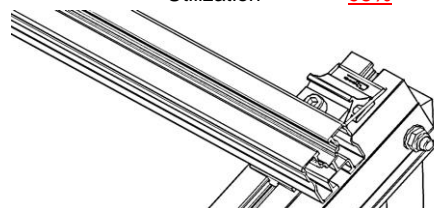
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.357 k
Allowable Uplift =	1.214 k
Utilization =	<u>29%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.058 k
Allowable Uplift =	1.116 k
Utilization =	<u>95%</u>



### 6.2 Bolted Connections

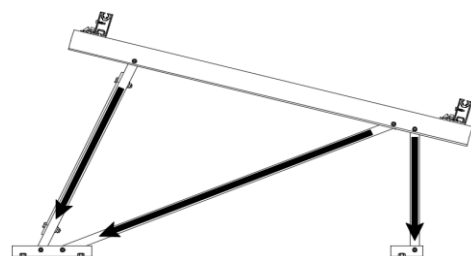
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.776 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>31%</u>

#### Diagonal Strut

Maximum Axial Load =	0.151 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.433 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>25%</u>

#### Bracing

Maximum Axial Load =	0.271 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.125 in
	<u>0.125 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$212.736$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102.00 \text{ in}$$

$$J = 0.427$$

$$231.168$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.4$$

#### 3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.778 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.45 \\ &20.4426 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$b/t = 4.29$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.45 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \end{aligned}$$

$$S1 = 1.37733$$

$$S2 = 1.2C_c$$

$$S2 = 79.2$$

$$\phi F_L = \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})]$$

$$\phi F_L = 29.9 \text{ ksi}$$

#### 3.4.15

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp}$$

$$S1 = 3.8$$

$$S2 = \frac{k_1 Bp}{5.1Dp}$$

$$S2 = 14.7$$

$$F_{UT} = (\phi b k_2 * \sqrt{BpE}) / (5.1b/t)$$

$$F_{UT} = 9.4 \text{ ksi}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$b/t = 24.46$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$F_{ST} = \phi b[Bp - 1.6Dp * b/t]$$

$$F_{ST} = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.16.2

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.9 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.469 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0      \...\PVMMini 60 Cell 1V 15° 110mph 30psf 8.5ft 7-10.rdb Page 20



RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 15° 110mph 30psf 8.5ft 7-10.r    Page 21



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	436.825	1	.125	6	.912	1	0	12	.002	1	0	15
30			min	-351.746	3	.029	15	-.406	5	-.001	1	0	3	0	6
31		16	max	436.922	1	.087	6	.912	1	0	12	.002	1	0	15
32			min	-351.674	3	.02	15	-.493	5	-.001	1	0	3	0	6
33		17	max	437.018	1	.053	10	.912	1	0	12	.002	1	0	15
34			min	-351.601	3	-.003	1	-.58	5	-.001	1	0	3	0	6
35		18	max	437.115	1	.028	10	.912	1	0	12	.002	1	0	15
36			min	-351.529	3	-.032	1	-.668	5	-.001	1	0	3	0	6
37		19	max	437.211	1	.004	10	.912	1	0	12	.002	1	0	15
38			min	-351.457	3	-.062	1	-.755	5	-.001	1	0	3	0	6
39	M3	1	max	32.303	10	1.807	6	-.031	12	0	5	.002	1	0	6
40			min	-130.842	1	.424	15	-1.518	4	0	1	0	12	0	15
41		2	max	32.248	10	1.629	6	-.031	12	0	5	.002	1	0	6
42			min	-130.909	1	.383	15	-1.384	4	0	1	0	12	0	15
43		3	max	32.192	10	1.451	6	-.031	12	0	5	.002	1	0	10
44			min	-130.976	1	.341	15	-1.251	4	0	1	0	12	0	1
45		4	max	32.136	10	1.273	6	-.031	12	0	5	.002	1	0	15
46			min	-131.043	1	.299	15	-1.117	4	0	1	0	15	0	1
47		5	max	32.08	10	1.095	6	-.031	12	0	5	.002	1	0	15
48			min	-131.11	1	.257	15	-.983	4	0	1	0	5	0	4
49		6	max	32.024	10	.917	6	-.031	12	0	5	.001	1	0	15
50			min	-131.177	1	.215	15	-.85	4	0	1	0	5	0	4
51		7	max	31.968	10	.739	6	-.031	12	0	5	.001	1	0	15
52			min	-131.244	1	.173	15	-.79	1	0	1	0	5	0	4
53		8	max	31.912	10	.561	6	-.031	12	0	5	.001	1	0	15
54			min	-131.311	1	.132	15	-.79	1	0	1	0	5	0	4
55		9	max	31.856	10	.383	6	-.031	12	0	5	0	1	0	15
56			min	-131.378	1	.09	15	-.79	1	0	1	0	5	-.001	4
57		10	max	31.8	10	.205	6	-.031	12	0	5	0	1	0	15
58			min	-131.445	1	.048	15	-.79	1	0	1	0	5	-.001	4
59		11	max	31.744	10	.031	10	-.019	15	0	5	0	1	0	15
60			min	-131.513	1	-.006	1	-.79	1	0	1	0	5	-.001	4
61		12	max	31.688	10	-.036	15	.104	5	0	5	0	1	0	15
62			min	-131.58	1	-.151	4	-.79	1	0	1	0	5	-.001	4
63		13	max	31.633	10	-.078	15	.237	5	0	5	0	1	0	15
64			min	-131.647	1	-.329	4	-.79	1	0	1	0	5	-.001	4
65		14	max	31.577	10	-.12	15	.371	5	0	5	0	1	0	15
66			min	-131.714	1	-.507	4	-.79	1	0	1	0	5	-.001	4
67		15	max	31.521	10	-.161	15	.504	5	0	5	0	12	0	15
68			min	-131.781	1	-.685	4	-.79	1	0	1	0	4	0	4
69		16	max	31.465	10	-.203	15	.638	5	0	5	0	12	0	15
70			min	-131.848	1	-.863	4	-.79	1	0	1	0	4	0	4
71		17	max	31.409	10	-.245	15	.771	5	0	5	0	12	0	15
72			min	-131.915	1	-1.041	4	-.79	1	0	1	0	1	0	4
73		18	max	31.353	10	-.287	15	.905	5	0	5	0	12	0	15
74			min	-131.982	1	-1.219	4	-.79	1	0	1	0	1	0	4
75		19	max	31.297	10	-.329	15	1.038	5	0	5	0	5	0	1
76			min	-132.049	1	-1.397	4	-.79	1	0	1	0	1	0	1
77	M4	1	max	595.067	1	0	1	-.114	12	0	1	0	5	0	1
78			min	-107.184	3	0	1	-36.046	4	0	1	0	1	0	1
79		2	max	595.131	1	0	1	-.114	12	0	1	0	12	0	1
80			min	-107.135	3	0	1	-36.102	4	0	1	-.003	4	0	1
81		3	max	595.196	1	0	1	-.114	12	0	1	0	12	0	1
82			min	-107.086	3	0	1	-36.158	4	0	1	-.006	4	0	1
83		4	max	595.261	1	0	1	-.114	12	0	1	0	12	0	1
84			min	-107.038	3	0	1	-36.214	4	0	1	-.01	4	0	1
85		5	max	595.326	1	0	1	-.114	12	0	1	0	12	0	1







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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1432.765	1	.121	2	.347	1	0	1	0	14	0	15
144		min	-1158.106	3	.006	9	-.349	5	0	5	0	3	0	6
145	16	max	1432.861	1	.094	10	.347	1	0	1	0	14	0	15
146		min	-1158.034	3	-.023	1	-.436	5	0	5	0	3	0	6
147	17	max	1432.957	1	.069	10	.347	1	0	1	0	14	0	15
148		min	-1157.961	3	-.053	1	-.524	5	0	5	0	3	0	6
149	18	max	1433.054	1	.045	10	.347	1	0	1	0	14	0	15
150		min	-1157.889	3	-.082	1	-.611	5	0	5	0	3	0	6
151	19	max	1433.15	1	.02	10	.347	1	0	1	0	14	0	15
152		min	-1157.817	3	-.112	1	-.698	5	0	5	0	3	0	6
153	M7	1	max	151.341	2	1.808	.015	1	0	2	0	4	0	4
154		min	-176.683	9	.43	15	-1.505	5	0	5	0	3	0	15
155	2	max	151.273	2	1.63	4	.015	1	0	2	0	4	0	2
156		min	-176.739	9	.388	15	-1.371	5	0	5	0	3	0	15
157	3	max	151.206	2	1.452	4	.015	1	0	2	0	4	0	2
158		min	-176.795	9	.346	15	-1.237	5	0	5	0	3	0	9
159	4	max	151.139	2	1.274	4	.015	1	0	2	0	14	0	10
160		min	-176.851	9	.304	15	-1.104	5	0	5	0	3	0	1
161	5	max	151.072	2	1.096	4	.015	1	0	2	0	2	0	15
162		min	-176.907	9	.262	15	-.97	5	0	5	0	5	0	1
163	6	max	151.005	2	.918	4	.015	1	0	2	0	2	0	15
164		min	-176.963	9	.22	15	-.837	5	0	5	0	5	0	6
165	7	max	150.938	2	.74	4	.015	1	0	2	0	2	0	15
166		min	-177.019	9	.179	15	-.703	5	0	5	0	5	0	6
167	8	max	150.871	2	.562	4	.015	1	0	2	0	2	0	15
168		min	-177.075	9	.137	15	-.57	5	0	5	0	5	0	6
169	9	max	150.804	2	.384	4	.015	1	0	2	0	2	0	15
170		min	-177.131	9	.095	15	-.436	5	0	5	0	5	-.001	6
171	10	max	150.737	2	.206	4	.015	1	0	2	0	2	0	15
172		min	-177.186	9	.053	15	-.303	5	0	5	0	5	-.001	6
173	11	max	150.67	2	.05	2	.015	1	0	2	0	2	0	15
174		min	-177.242	9	-.023	9	-.169	5	0	5	0	5	-.001	6
175	12	max	150.603	2	-.031	15	.015	1	0	2	0	2	0	15
176		min	-177.298	9	-.16	1	-.036	5	0	5	0	5	-.001	6
177	13	max	150.535	2	-.072	15	.101	4	0	2	0	2	0	15
178		min	-177.354	9	-.328	6	-.007	3	0	5	0	5	-.001	6
179	14	max	150.468	2	-.114	15	.234	4	0	2	0	2	0	15
180		min	-177.41	9	-.506	6	-.007	3	0	5	0	5	-.001	6
181	15	max	150.401	2	-.156	15	.368	4	0	2	0	2	0	15
182		min	-177.466	9	-.684	6	-.007	3	0	5	0	5	0	6
183	16	max	150.334	2	-.198	15	.502	4	0	2	0	2	0	15
184		min	-177.522	9	-.862	6	-.007	3	0	5	0	5	0	6
185	17	max	150.267	2	-.24	15	.635	4	0	2	0	2	0	15
186		min	-177.578	9	-1.04	6	-.007	3	0	5	0	5	0	6
187	18	max	150.2	2	-.282	15	.769	4	0	2	0	2	0	15
188		min	-177.634	9	-1.218	6	-.007	3	0	5	0	5	0	6
189	19	max	150.133	2	-.324	15	.902	4	0	2	0	2	0	1
190		min	-177.69	9	-1.396	6	-.007	3	0	5	0	5	0	1
191	M8	1	max	1774.79	1	0	.72	1	0	1	0	4	0	1
192		min	-375.047	3	0	1	-36.375	4	0	1	0	1	0	1
193	2	max	1774.855	1	0	1	.72	1	0	1	0	1	0	1
194		min	-374.999	3	0	1	-36.431	4	0	1	-.003	4	0	1
195	3	max	1774.919	1	0	1	.72	1	0	1	0	1	0	1
196		min	-374.95	3	0	1	-36.487	4	0	1	-.006	4	0	1
197	4	max	1774.984	1	0	1	.72	1	0	1	0	1	0	1
198		min	-374.902	3	0	1	-36.543	4	0	1	-.01	4	0	1
199	5	max	1775.049	1	0	1	.72	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-374.853	3	0	1	-36.6	4	0	1	-.013	4	0	1
201		6	max	1775.113	1	0	1	.72	1	0	1	0	1	0	1
202			min	-374.805	3	0	1	-36.656	4	0	1	-.016	4	0	1
203		7	max	1775.178	1	0	1	.72	1	0	1	0	1	0	1
204			min	-374.756	3	0	1	-36.712	4	0	1	-.02	4	0	1
205		8	max	1775.243	1	0	1	.72	1	0	1	0	1	0	1
206			min	-374.708	3	0	1	-36.768	4	0	1	-.023	4	0	1
207		9	max	1775.307	1	0	1	.72	1	0	1	0	1	0	1
208			min	-374.659	3	0	1	-36.824	4	0	1	-.026	4	0	1
209		10	max	1775.372	1	0	1	.72	1	0	1	0	1	0	1
210			min	-374.611	3	0	1	-36.88	4	0	1	-.029	4	0	1
211		11	max	1775.437	1	0	1	.72	1	0	1	0	1	0	1
212			min	-374.562	3	0	1	-36.936	4	0	1	-.033	4	0	1
213		12	max	1775.502	1	0	1	.72	1	0	1	0	1	0	1
214			min	-374.514	3	0	1	-36.992	4	0	1	-.036	4	0	1
215		13	max	1775.566	1	0	1	.72	1	0	1	0	1	0	1
216			min	-374.465	3	0	1	-37.048	4	0	1	-.039	4	0	1
217		14	max	1775.631	1	0	1	.72	1	0	1	0	1	0	1
218			min	-374.417	3	0	1	-37.104	4	0	1	-.043	4	0	1
219		15	max	1775.696	1	0	1	.72	1	0	1	0	1	0	1
220			min	-374.368	3	0	1	-37.16	4	0	1	-.046	4	0	1
221		16	max	1775.76	1	0	1	.72	1	0	1	0	1	0	1
222			min	-374.319	3	0	1	-37.216	4	0	1	-.049	4	0	1
223		17	max	1775.825	1	0	1	.72	1	0	1	.001	1	0	1
224			min	-374.271	3	0	1	-37.272	4	0	1	-.053	4	0	1
225		18	max	1775.89	1	0	1	.72	1	0	1	.001	1	0	1
226			min	-374.222	3	0	1	-37.329	4	0	1	-.056	4	0	1
227		19	max	1775.955	1	0	1	.72	1	0	1	.001	1	0	1
228			min	-374.174	3	0	1	-37.385	4	0	1	-.059	4	0	1
229	M10	1	max	446.511	1	.68	4	1.254	4	.001	1	0	5	0	1
230			min	-344.015	3	.171	15	-.13	1	-.002	5	0	3	0	1
231		2	max	446.607	1	.642	4	1.166	4	.001	1	0	4	0	15
232			min	-343.942	3	.162	15	-.13	1	-.002	5	0	3	0	4
233		3	max	446.704	1	.604	4	1.079	4	.001	1	0	4	0	15
234			min	-343.87	3	.153	15	-.13	1	-.002	5	0	3	0	4
235		4	max	446.8	1	.566	4	.992	4	.001	1	0	4	0	15
236			min	-343.798	3	.144	15	-.13	1	-.002	5	0	3	0	4
237		5	max	446.897	1	.529	4	.904	4	.001	1	0	4	0	15
238			min	-343.725	3	.135	15	-.13	1	-.002	5	0	1	0	4
239		6	max	446.993	1	.491	4	.817	4	.001	1	0	4	0	15
240			min	-343.653	3	.126	15	-.13	1	-.002	5	0	1	0	4
241		7	max	447.089	1	.453	4	.73	4	.001	1	.001	4	0	15
242			min	-343.581	3	.117	15	-.13	1	-.002	5	0	1	0	4
243		8	max	447.186	1	.415	4	.642	4	.001	1	.001	4	0	15
244			min	-343.509	3	.108	15	-.13	1	-.002	5	0	1	0	4
245		9	max	447.282	1	.377	4	.555	4	.001	1	.001	4	0	15
246			min	-343.436	3	.1	15	-.13	1	-.002	5	0	1	0	4
247		10	max	447.378	1	.339	4	.468	4	.001	1	.001	4	0	15
248			min	-343.364	3	.091	15	-.13	1	-.002	5	0	1	0	4
249		11	max	447.475	1	.302	4	.38	4	.001	1	.001	4	0	15
250			min	-343.292	3	.082	15	-.13	1	-.002	5	0	1	0	4
251		12	max	447.571	1	.264	4	.293	4	.001	1	.001	4	0	15
252			min	-343.22	3	.073	15	-.13	1	-.002	5	0	1	0	4
253		13	max	447.667	1	.226	4	.206	4	.001	1	.001	4	0	15
254			min	-343.147	3	.052	1	-.13	1	-.002	5	0	1	0	4
255		14	max	447.764	1	.188	4	.118	4	.001	1	.001	4	0	15
256			min	-343.075	3	.023	1	-.13	1	-.002	5	0	1	0	4



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	447.86	1	.15	4	.031	4	.001	1	.001	4	0	15
258		min	-343.003	3	-.007	1	-.13	1	-.002	5	0	1	0	4
259	16	max	447.957	1	.112	4	-.022	14	.001	1	.001	4	0	15
260		min	-342.93	3	-.036	1	-.13	1	-.002	5	0	1	0	4
261	17	max	448.053	1	.075	4	-.023	12	.001	1	.001	4	0	15
262		min	-342.858	3	-.066	1	-.158	5	-.002	5	0	1	0	4
263	18	max	448.149	1	.051	3	-.023	12	.001	1	.001	4	0	15
264		min	-342.786	3	-.095	1	-.246	5	-.002	5	0	1	0	4
265	19	max	448.246	1	.028	3	-.023	12	.001	1	.001	4	0	15
266		min	-342.714	3	-.125	1	-.333	5	-.002	5	0	1	0	4
267	M11	1	max	31.731	10	1.806	.932	1	.002	4	.002	5	0	6
268		min	-130.66	1	.423	15	-1.103	5	0	10	-.002	1	0	15
269	2	max	31.675	10	1.628	6	.932	1	.002	4	.001	5	0	2
270		min	-130.727	1	.382	15	-.97	5	0	10	-.002	1	0	15
271	3	max	31.619	10	1.45	6	.932	1	.002	4	.001	5	0	2
272		min	-130.794	1	.34	15	-.836	5	0	10	-.002	1	0	3
273	4	max	31.563	10	1.272	6	.932	1	.002	4	.001	5	0	15
274		min	-130.861	1	.298	15	-.703	5	0	10	-.002	1	0	4
275	5	max	31.507	10	1.094	6	.932	1	.002	4	0	5	0	15
276		min	-130.928	1	.256	15	-.569	5	0	10	-.001	1	0	4
277	6	max	31.451	10	.916	6	.932	1	.002	4	0	5	0	15
278		min	-130.995	1	.214	15	-.436	5	0	10	-.001	1	0	4
279	7	max	31.395	10	.738	6	.932	1	.002	4	0	5	0	15
280		min	-131.062	1	.172	15	-.302	5	0	10	-.001	1	0	4
281	8	max	31.339	10	.56	6	.932	1	.002	4	0	5	0	15
282		min	-131.129	1	.13	15	-.169	5	0	10	0	1	0	4
283	9	max	31.283	10	.382	6	.932	1	.002	4	0	5	0	15
284		min	-131.196	1	.089	15	-.035	5	0	10	0	1	-.001	4
285	10	max	31.227	10	.204	6	.932	1	.002	4	0	5	0	15
286		min	-131.263	1	.047	15	.022	12	0	10	0	1	-.001	4
287	11	max	31.172	10	.049	2	.932	1	.002	4	0	5	0	15
288		min	-131.331	1	.003	3	.022	12	0	10	0	1	-.001	4
289	12	max	31.116	10	-.037	15	.932	1	.002	4	0	5	0	15
290		min	-131.398	1	-.153	4	.022	12	0	10	0	2	-.001	4
291	13	max	31.06	10	-.079	15	.932	1	.002	4	0	4	0	15
292		min	-131.465	1	-.331	4	.022	12	0	10	0	10	-.001	4
293	14	max	31.004	10	-.121	15	.932	1	.002	4	.001	4	0	15
294		min	-131.532	1	-.509	4	.022	12	0	10	0	10	-.001	4
295	15	max	30.948	10	-.162	15	.944	4	.002	4	.001	4	0	15
296		min	-131.599	1	-.687	4	.022	12	0	10	0	10	0	4
297	16	max	30.892	10	-.204	15	1.077	4	.002	4	.001	4	0	15
298		min	-131.666	1	-.865	4	.022	12	0	10	0	10	0	4
299	17	max	30.836	10	-.246	15	1.211	4	.002	4	.002	4	0	15
300		min	-131.733	1	-1.043	4	.022	12	0	10	0	10	0	4
301	18	max	30.78	10	-.288	15	1.345	4	.002	4	.002	4	0	15
302		min	-131.8	1	-1.221	4	.022	12	0	10	0	10	0	4
303	19	max	30.724	10	-.33	15	1.478	4	.002	4	.002	4	0	1
304		min	-131.867	1	-1.399	4	.022	12	0	10	0	10	0	1
305	M12	1	max	594.972	1	0	1	3.718	1	0	1	0	4	1
306		min	-106.776	3	0	1	-33.174	5	0	1	0	3	0	1
307	2	max	595.037	1	0	1	3.718	1	0	1	0	1	0	1
308		min	-106.728	3	0	1	-33.23	5	0	1	-.003	5	0	1
309	3	max	595.102	1	0	1	3.718	1	0	1	0	1	0	1
310		min	-106.679	3	0	1	-33.286	5	0	1	-.006	5	0	1
311	4	max	595.166	1	0	1	3.718	1	0	1	.001	1	0	1
312		min	-106.63	3	0	1	-33.342	5	0	1	-.009	5	0	1
313	5	max	595.231	1	0	1	3.718	1	0	1	.001	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-106.582	3	0	1	-33.398	5	0	1	-.012	5	0	1
315	6	max	595.296	1	0	1	3.718	1	0	1	.002	1	0	1
316		min	-106.533	3	0	1	-33.454	5	0	1	-.015	5	0	1
317	7	max	595.361	1	0	1	3.718	1	0	1	.002	1	0	1
318		min	-106.485	3	0	1	-33.51	5	0	1	-.018	5	0	1
319	8	max	595.425	1	0	1	3.718	1	0	1	.002	1	0	1
320		min	-106.436	3	0	1	-33.566	5	0	1	-.021	5	0	1
321	9	max	595.49	1	0	1	3.718	1	0	1	.003	1	0	1
322		min	-106.388	3	0	1	-33.622	5	0	1	-.024	5	0	1
323	10	max	595.555	1	0	1	3.718	1	0	1	.003	1	0	1
324		min	-106.339	3	0	1	-33.678	5	0	1	-.027	5	0	1
325	11	max	595.619	1	0	1	3.718	1	0	1	.003	1	0	1
326		min	-106.291	3	0	1	-33.734	5	0	1	-.03	5	0	1
327	12	max	595.684	1	0	1	3.718	1	0	1	.004	1	0	1
328		min	-106.242	3	0	1	-33.79	5	0	1	-.033	5	0	1
329	13	max	595.749	1	0	1	3.718	1	0	1	.004	1	0	1
330		min	-106.194	3	0	1	-33.846	5	0	1	-.036	5	0	1
331	14	max	595.814	1	0	1	3.718	1	0	1	.004	1	0	1
332		min	-106.145	3	0	1	-33.903	5	0	1	-.039	5	0	1
333	15	max	595.878	1	0	1	3.718	1	0	1	.005	1	0	1
334		min	-106.097	3	0	1	-33.959	5	0	1	-.042	5	0	1
335	16	max	595.943	1	0	1	3.718	1	0	1	.005	1	0	1
336		min	-106.048	3	0	1	-34.015	5	0	1	-.045	5	0	1
337	17	max	596.008	1	0	1	3.718	1	0	1	.005	1	0	1
338		min	-106	3	0	1	-34.071	5	0	1	-.048	5	0	1
339	18	max	596.072	1	0	1	3.718	1	0	1	.006	1	0	1
340		min	-105.951	3	0	1	-34.127	5	0	1	-.051	5	0	1
341	19	max	596.137	1	0	1	3.718	1	0	1	.006	1	0	1
342		min	-105.903	3	0	1	-34.183	5	0	1	-.054	5	0	1
343	M1	1	max	118.469	1	330.551	3	-2.521	12	0	.144	1	.015	1
344		min	3.874	12	-434.12	1	-73.133	1	0	3	.005	12	-.009	3
345	2	max	118.542	1	330.348	3	-2.521	12	0	1	.128	1	.109	1
346		min	3.91	12	-434.39	1	-73.133	1	0	3	.005	12	-.081	3
347	3	max	134.269	1	7.176	9	-2.548	12	0	5	.111	1	.201	1
348		min	-6.05	3	-22.43	3	-72.712	1	0	1	.004	12	-.151	3
349	4	max	134.342	1	6.951	9	-2.548	12	0	5	.096	1	.201	1
350		min	-5.995	3	-22.632	3	-72.712	1	0	1	.004	12	-.146	3
351	5	max	134.414	1	6.726	9	-2.548	12	0	5	.08	1	.201	1
352		min	-5.941	3	-22.834	3	-72.712	1	0	1	.003	12	-.142	3
353	6	max	134.486	1	6.501	9	-2.548	12	0	5	.064	1	.201	1
354		min	-5.887	3	-23.037	3	-72.712	1	0	1	.003	12	-.137	3
355	7	max	134.558	1	6.277	9	-2.548	12	0	5	.048	1	.201	1
356		min	-5.833	3	-23.239	3	-72.712	1	0	1	.002	12	-.132	3
357	8	max	134.631	1	6.052	9	-2.548	12	0	5	.032	1	.201	1
358		min	-5.778	3	-23.441	3	-72.712	1	0	1	.001	12	-.126	3
359	9	max	134.703	1	5.827	9	-2.548	12	0	5	.017	1	.201	1
360		min	-5.724	3	-23.644	3	-72.712	1	0	1	0	12	-.121	3
361	10	max	134.775	1	5.602	9	-2.548	12	0	5	.003	4	.201	1
362		min	-5.67	3	-23.846	3	-72.712	1	0	1	0	10	-.116	3
363	11	max	134.847	1	5.377	9	-2.548	12	0	5	0	15	.201	1
364		min	-5.616	3	-24.048	3	-72.712	1	0	1	-.015	1	-.111	3
365	12	max	134.92	1	5.153	9	-2.548	12	0	5	0	12	.201	1
366		min	-5.562	3	-24.25	3	-72.712	1	0	1	-.031	1	-.106	3
367	13	max	134.992	1	4.928	9	-2.548	12	0	5	-.001	12	.202	1
368		min	-5.507	3	-24.453	3	-72.712	1	0	1	-.046	1	-.1	3
369	14	max	135.064	1	4.703	9	-2.548	12	0	5	-.002	12	.202	1
370		min	-5.453	3	-24.655	3	-72.712	1	0	1	-.062	1	-.095	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	135.137	1	4.478	9	-2.548	12	0	5	-.002	12	.203	1
372		min	-5.399	3	-24.857	3	-72.712	1	0	1	-.078	1	-.09	3
373	16	max	66.481	2	8.019	10	-2.58	12	0	1	-.003	12	.204	1
374		min	-33.314	3	-89.315	1	-73.415	1	0	4	-.095	1	-.084	3
375	17	max	66.554	2	7.794	10	-2.58	12	0	1	-.004	12	.223	1
376		min	-33.26	3	-89.585	1	-73.415	1	0	4	-.111	1	-.073	3
377	18	max	-3.652	12	484.039	1	-2.695	12	0	5	-.004	12	.12	1
378		min	-118.042	1	-154.206	3	-75.136	1	0	1	-.127	1	-.04	3
379	19	max	-3.616	12	483.769	1	-2.695	12	0	5	-.005	12	.016	1
380		min	-117.97	1	-154.408	3	-75.136	1	0	1	-.143	1	-.007	3
381	M5	1	max	259.127	1	1093.598	3	-.085	10	0	.05	4	.019	3
382		min	5.066	15	-1437.035	1	-30.364	4	0	5	0	10	-.029	1
383	2	max	259.199	1	1093.395	3	-.085	10	0	1	.043	4	.282	1
384		min	5.088	15	-1437.304	1	-30.122	4	0	5	-.002	3	-.218	3
385	3	max	309.298	1	10.668	9	2.446	3	0	3	.036	4	.588	1
386		min	-29.776	3	-74.129	3	-26.702	4	0	4	-.007	3	-.451	3
387	4	max	309.37	1	10.444	9	2.446	3	0	3	.03	4	.591	1
388		min	-29.722	3	-74.331	3	-26.46	4	0	4	-.006	3	-.434	3
389	5	max	309.442	1	10.219	9	2.446	3	0	3	.025	4	.594	1
390		min	-29.668	3	-74.533	3	-26.218	4	0	4	-.006	3	-.418	3
391	6	max	309.515	1	9.994	9	2.446	3	0	3	.019	4	.597	1
392		min	-29.614	3	-74.736	3	-25.976	4	0	4	-.005	3	-.402	3
393	7	max	309.587	1	9.769	9	2.446	3	0	3	.013	4	.6	1
394		min	-29.559	3	-74.938	3	-25.734	4	0	4	-.005	1	-.386	3
395	8	max	309.659	1	9.545	9	2.446	3	0	3	.008	4	.603	1
396		min	-29.505	3	-75.14	3	-25.492	4	0	4	-.004	1	-.37	3
397	9	max	309.731	1	9.32	9	2.446	3	0	3	.002	5	.606	1
398		min	-29.451	3	-75.342	3	-25.25	4	0	4	-.004	1	-.353	3
399	10	max	309.804	1	9.095	9	2.446	3	0	3	0	10	.61	1
400		min	-29.397	3	-75.545	3	-25.008	4	0	4	-.003	1	-.337	3
401	11	max	309.876	1	8.87	9	2.446	3	0	3	0	10	.613	1
402		min	-29.343	3	-75.747	3	-24.766	4	0	4	-.008	4	-.321	3
403	12	max	309.948	1	8.646	9	2.446	3	0	3	0	10	.616	1
404		min	-29.288	3	-75.949	3	-24.524	4	0	4	-.014	4	-.304	3
405	13	max	310.02	1	8.421	9	2.446	3	0	3	0	10	.62	1
406		min	-29.234	3	-76.152	3	-24.282	4	0	4	-.019	4	-.288	3
407	14	max	310.093	1	8.196	9	2.446	3	0	3	0	10	.623	1
408		min	-29.18	3	-76.354	3	-24.04	4	0	4	-.024	4	-.271	3
409	15	max	310.165	1	7.971	9	2.446	3	0	3	0	10	.627	1
410		min	-29.126	3	-76.556	3	-23.798	4	0	4	-.03	4	-.254	3
411	16	max	249.019	2	48.417	10	2.426	3	0	1	0	3	.631	1
412		min	-108.13	3	-145.294	3	-22.668	4	0	4	-.035	4	-.237	3
413	17	max	249.092	2	48.192	10	2.426	3	0	1	0	3	.655	1
414		min	-108.075	3	-145.497	3	-22.426	4	0	4	-.04	4	-.206	3
415	18	max	-7.51	12	1596.696	1	2.251	1	0	4	.001	3	.315	1
416		min	-259.87	1	-508.696	3	-57.002	5	0	1	-.052	4	-.097	3
417	19	max	-7.474	12	1596.426	1	2.251	1	0	4	.002	3	.014	3
418		min	-259.798	1	-508.899	3	-56.76	5	0	1	-.064	4	-.031	1
419	M9	1	max	117.925	1	330.541	3	232.111	4	0	3	0	.015	1
420		min	1.682	15	-434.101	1	5.866	10	0	1	-.144	1	-.009	3
421	2	max	117.997	1	330.339	3	232.353	4	0	3	.047	5	.109	1
422		min	1.704	15	-434.371	1	5.866	10	0	1	-.123	1	-.081	3
423	3	max	134.342	1	7.156	9	68.407	1	0	1	.091	5	.201	1
424		min	-5.606	3	-22.377	3	-35.971	5	0	12	-.1	1	-.151	3
425	4	max	134.414	1	6.931	9	68.407	1	0	1	.083	5	.201	1
426		min	-5.552	3	-22.579	3	-35.729	5	0	12	-.085	1	-.146	3
427	5	max	134.487	1	6.706	9	68.407	1	0	1	.075	5	.201	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-5.498	3	-22.781	3	-35.487	5	0	12	-.07	1	-.141	3
429		6	max	134.559	1	6.481	9	68.407	1	0	1	.068	5	.201	1
430			min	-5.443	3	-22.984	3	-35.245	5	0	12	-.056	1	-.137	3
431		7	max	134.631	1	6.256	9	68.407	1	0	1	.06	5	.201	1
432			min	-5.389	3	-23.186	3	-35.003	5	0	12	-.041	1	-.131	3
433		8	max	134.703	1	6.032	9	68.407	1	0	1	.052	5	.201	1
434			min	-5.335	3	-23.388	3	-34.761	5	0	12	-.026	1	-.126	3
435		9	max	134.776	1	5.807	9	68.407	1	0	1	.045	5	.201	1
436			min	-5.281	3	-23.59	3	-34.519	5	0	12	-.011	1	-.121	3
437		10	max	134.848	1	5.582	9	68.407	1	0	1	.038	4	.201	1
438			min	-5.227	3	-23.793	3	-34.277	5	0	12	0	10	-.116	3
439		11	max	134.92	1	5.357	9	68.407	1	0	1	.033	4	.201	1
440			min	-5.172	3	-23.995	3	-34.035	5	0	12	.001	10	-.111	3
441		12	max	134.993	1	5.133	9	68.407	1	0	1	.033	1	.201	1
442			min	-5.118	3	-24.197	3	-33.793	5	0	12	.002	10	-.106	3
443		13	max	135.065	1	4.908	9	68.407	1	0	1	.048	1	.202	1
444			min	-5.064	3	-24.4	3	-33.551	5	0	12	.003	12	-.101	3
445		14	max	135.137	1	4.683	9	68.407	1	0	1	.063	1	.202	1
446			min	-5.01	3	-24.602	3	-33.309	5	0	12	.003	12	-.095	3
447		15	max	135.209	1	4.458	9	68.407	1	0	1	.078	1	.203	1
448			min	-4.956	3	-24.804	3	-33.067	5	0	12	0	15	-.09	3
449		16	max	66.679	2	7.66	10	69.282	1	0	10	.095	1	.204	1
450			min	-33.362	3	-89.224	1	-31.532	5	0	4	-.004	5	-.084	3
451		17	max	66.751	2	7.436	10	69.282	1	0	10	.11	1	.223	1
452			min	-33.308	3	-89.494	1	-31.29	5	0	4	-.01	5	-.073	3
453		18	max	3.717	5	484.039	1	72.897	1	0	1	.125	1	.12	1
454			min	-117.825	1	-154.205	3	-64.117	5	0	3	-.024	5	-.04	3
455		19	max	3.751	5	483.769	1	72.897	1	0	1	.141	1	.016	1
456			min	-117.753	1	-154.407	3	-63.875	5	0	3	-.038	5	-.007	3
457	M13	1	max	232.12	4	433.506	1	-1.682	15	.015	1	.144	1	0	1
458			min	5.868	10	-330.531	3	-117.912	1	-.009	3	0	15	0	3
459		2	max	222.768	4	305.748	1	-.839	15	.015	1	.045	1	.266	3
460			min	5.868	10	-233.057	3	-90.383	1	-.009	3	-.002	5	-.349	1
461		3	max	213.417	4	177.99	1	.003	15	.015	1	0	3	.44	3
462			min	5.868	10	-135.583	3	-62.854	1	-.009	3	-.027	1	-.577	1
463		4	max	204.065	4	50.232	1	1.199	5	.015	1	-.001	15	.522	3
464			min	5.868	10	-38.109	3	-35.325	1	-.009	3	-.073	1	-.685	1
465		5	max	194.714	4	59.366	3	2.502	5	.015	1	0	15	.512	3
466			min	5.868	10	-77.526	1	-7.796	1	-.009	3	-.094	1	-.672	1
467		6	max	185.362	4	156.84	3	19.733	1	.015	1	.003	5	.41	3
468			min	5.868	10	-205.284	1	.315	12	-.009	3	-.088	1	-.539	1
469		7	max	176.011	4	254.314	3	47.262	1	.015	1	.007	5	.216	3
470			min	5.868	10	-333.042	1	1.137	12	-.009	3	-.056	1	-.285	1
471		8	max	166.659	4	351.788	3	74.791	1	.015	1	.013	4	.09	1
472			min	5.868	10	-460.8	1	1.959	12	-.009	3	0	12	-.07	3
473		9	max	157.308	4	449.262	3	102.32	1	.015	1	.085	1	.586	1
474			min	5.868	10	-588.558	1	2.781	12	-.009	3	.002	12	-.449	3
475		10	max	147.956	4	546.736	3	129.849	1	.011	2	.195	1	1.202	1
476			min	5.868	10	-716.316	1	3.602	12	-.015	1	.005	12	-.919	3
477		11	max	108.358	4	588.558	1	2.31	5	.009	3	.081	1	.586	1
478			min	2.521	12	-449.262	3	-101.772	1	-.015	1	-.019	5	-.449	3
479		12	max	99.006	4	460.8	1	3.614	5	.009	3	0	10	.09	1
480			min	2.521	12	-351.788	3	-74.243	1	-.015	1	-.017	4	-.07	3
481		13	max	89.655	4	333.042	1	4.917	5	.009	3	-.003	12	.216	3
482			min	2.521	12	-254.314	3	-46.714	1	-.015	1	-.059	1	-.285	1
483		14	max	80.303	4	205.284	1	6.22	5	.009	3	-.003	12	.41	3
484			min	2.521	12	-156.84	3	-19.185	1	-.015	1	-.09	1	-.539	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	73.344	1	77.526	1	9.267	4	.009	3	0	15	.512	3
486			min	2.521	12	-59.365	3	.472	10	-.015	1	-.095	1	-.672	1
487		16	max	73.344	1	38.109	3	35.873	1	.009	3	.007	5	.522	3
488			min	2.521	12	-50.232	1	1.408	12	-.015	1	-.074	1	-.685	1
489		17	max	73.344	1	135.583	3	63.402	1	.009	3	.016	5	.44	3
490			min	2.521	12	-177.99	1	2.23	12	-.015	1	-.028	1	-.577	1
491		18	max	73.344	1	233.057	3	90.931	1	.009	3	.045	1	.266	3
492			min	2.521	12	-305.748	1	3.052	12	-.015	1	.002	12	-.349	1
493		19	max	73.344	1	330.531	3	118.46	1	.009	3	.144	1	0	1
494			min	2.521	12	-433.506	1	3.874	12	-.015	1	.005	12	0	3
495	M16	1	max	63.862	5	484.398	1	3.751	5	.007	3	.141	1	0	1
496			min	-72.668	1	-154.422	3	-117.763	1	-.016	1	-.038	5	0	3
497		2	max	54.511	5	341.628	1	5.054	5	.007	3	.043	1	.124	3
498			min	-72.668	1	-108.987	3	-90.234	1	-.016	1	-.034	5	-.39	1
499		3	max	45.159	5	198.859	1	6.357	5	.007	3	0	12	.206	3
500			min	-72.668	1	-63.552	3	-62.705	1	-.016	1	-.034	4	-.645	1
501		4	max	35.808	5	56.089	1	7.661	5	.007	3	-.002	12	.244	3
502			min	-72.668	1	-18.116	3	-35.176	1	-.016	1	-.075	1	-.766	1
503		5	max	26.456	5	27.319	3	8.964	5	.007	3	-.003	12	.24	3
504			min	-72.668	1	-86.68	1	-7.647	1	-.016	1	-.096	1	-.751	1
505		6	max	17.105	5	72.754	3	19.882	1	.007	3	-.003	12	.193	3
506			min	-72.668	1	-229.45	1	.403	12	-.016	1	-.09	1	-.602	1
507		7	max	7.753	5	118.189	3	47.411	1	.007	3	.005	5	.103	3
508			min	-72.668	1	-372.219	1	1.225	12	-.016	1	-.058	1	-.318	1
509		8	max	-1.005	15	163.624	3	74.94	1	.007	3	.017	4	.101	1
510			min	-72.668	1	-514.989	1	2.047	12	-.016	1	-.001	3	-.03	3
511		9	max	-1.348	12	209.059	3	102.469	1	.007	3	.083	1	.655	1
512			min	-72.668	1	-657.758	1	2.869	12	-.016	1	.001	12	-.206	3
513		10	max	36.483	5	-17.625	15	129.998	1	.006	14	.193	1	1.344	1
514			min	-74.936	1	-800.528	1	-5.66	3	-.016	1	.006	12	-.425	3
515		11	max	27.131	5	657.758	1	2.401	5	.016	1	.084	1	.655	1
516			min	-74.936	1	-209.059	3	-102.251	1	-.007	3	-.018	5	-.206	3
517		12	max	17.78	5	514.989	1	3.704	5	.016	1	0	2	.101	1
518			min	-74.936	1	-163.624	3	-74.722	1	-.007	3	-.015	4	-.03	3
519		13	max	8.428	5	372.219	1	5.007	5	.016	1	-.002	12	.103	3
520			min	-74.936	1	-118.189	3	-47.193	1	-.007	3	-.057	1	-.318	1
521		14	max	-.521	15	229.45	1	6.31	5	.016	1	-.003	12	.193	3
522			min	-74.936	1	-72.754	3	-19.664	1	-.007	3	-.089	1	-.602	1
523		15	max	-2.694	12	86.68	1	9.328	4	.016	1	.001	5	.24	3
524			min	-74.936	1	-27.319	3	.327	12	-.007	3	-.094	1	-.751	1
525		16	max	-2.694	12	18.116	3	35.394	1	.016	1	.009	5	.244	3
526			min	-74.936	1	-56.089	1	1.149	12	-.007	3	-.074	1	-.766	1
527		17	max	-2.694	12	63.552	3	62.923	1	.016	1	.018	5	.206	3
528			min	-74.936	1	-198.859	1	1.971	12	-.007	3	-.028	1	-.645	1
529		18	max	-2.694	12	108.987	3	90.452	1	.016	1	.045	1	.124	3
530			min	-74.936	1	-341.628	1	2.793	12	-.007	3	.002	12	-.39	1
531		19	max	-2.694	12	154.422	3	117.981	1	.016	1	.143	1	0	1
532			min	-74.936	1	-484.398	1	3.615	12	-.007	3	.005	12	0	5
533	M15	1	max	0	4	2.304	1	.02	3	0	1	0	1	0	1
534			min	-26.929	1	0	4	-.029	1	0	3	0	3	0	1
535		2	max	0	4	2.048	1	.02	3	0	1	0	1	0	4
536			min	-27.001	1	0	4	-.029	1	0	3	0	3	-.001	1
537		3	max	0	4	1.792	1	.02	3	0	1	0	1	0	4
538			min	-27.073	1	0	4	-.029	1	0	3	0	3	-.002	1
539		4	max	0	4	1.536	1	.02	3	0	1	0	1	0	4
540			min	-27.145	1	0	4	-.029	1	0	3	0	3	-.003	1
541		5	max	0	4	1.28	1	.02	3	0	1	0	1	0	4



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-27.217	1	0	4	-.029	1	0	3	0	3	-.004	1
543		6	max	0	4	1.024	1	.02	3	0	1	0	1	0	4
544			min	-27.289	1	0	4	-.029	1	0	3	0	3	-.004	1
545		7	max	0	4	.768	1	.02	3	0	1	0	3	0	4
546			min	-27.361	1	0	4	-.029	1	0	3	0	1	-.005	1
547		8	max	0	4	.512	1	.02	3	0	1	0	3	0	4
548			min	-27.433	1	0	4	-.029	1	0	3	0	1	-.005	1
549		9	max	0	4	.256	1	.02	3	0	1	0	3	0	4
550			min	-27.505	1	0	4	-.029	1	0	3	0	1	-.005	1
551		10	max	0	4	0	1	.02	3	0	1	0	3	0	4
552			min	-27.577	1	0	1	-.029	1	0	3	0	1	-.005	1
553		11	max	0	4	0	4	.02	3	0	1	0	3	0	4
554			min	-27.649	1	-.256	2	-.029	1	0	3	0	1	-.005	1
555		12	max	0	4	0	4	.02	3	0	1	0	3	0	4
556			min	-27.721	1	-.512	2	-.029	1	0	3	0	1	-.005	1
557		13	max	0	4	0	4	.02	3	0	1	0	3	0	4
558			min	-27.793	1	-.768	2	-.029	1	0	3	0	1	-.005	1
559		14	max	0	4	0	4	.02	3	0	1	0	3	0	4
560			min	-27.865	1	-1.024	2	-.029	1	0	3	0	1	-.004	1
561		15	max	0	4	0	4	.02	3	0	1	0	3	0	4
562			min	-27.937	1	-1.28	2	-.029	1	0	3	0	1	-.004	1
563		16	max	0	4	0	4	.02	3	0	1	0	3	0	4
564			min	-28.009	1	-1.536	2	-.029	1	0	3	0	1	-.003	1
565		17	max	0	4	0	4	.02	3	0	1	0	3	0	4
566			min	-28.081	1	-1.792	2	-.029	1	0	3	0	1	-.002	1
567		18	max	0	4	0	4	.02	3	0	1	0	3	0	4
568			min	-28.152	1	-2.048	2	-.029	1	0	3	0	1	-.001	1
569		19	max	0	4	0	4	.02	3	0	1	0	3	0	1
570			min	-28.224	1	-2.304	2	-.029	1	0	3	0	1	0	1
571	M16A	1	max	-.796	10	3.515	4	.207	4	0	3	0	3	0	1
572			min	-268.248	4	1.071	15	-.008	3	0	1	0	4	0	1
573		2	max	-.736	10	3.125	4	.187	4	0	3	0	3	0	15
574			min	-268.377	4	.952	15	-.008	3	0	1	0	4	-.002	4
575		3	max	-.676	10	2.734	4	.168	4	0	3	0	3	0	15
576			min	-268.506	4	.833	15	-.008	3	0	1	0	4	-.003	4
577		4	max	-.616	10	2.344	4	.148	4	0	3	0	3	-.001	15
578			min	-268.634	4	.714	15	-.008	3	0	1	0	4	-.004	4
579		5	max	-.556	10	1.953	4	.128	4	0	3	0	3	-.002	15
580			min	-268.763	4	.595	15	-.008	3	0	1	0	1	-.005	4
581		6	max	-.496	10	1.562	4	.108	4	0	3	0	5	-.002	15
582			min	-268.892	4	.476	15	-.008	3	0	1	0	1	-.006	4
583		7	max	-.436	10	1.172	4	.088	4	0	3	0	5	-.002	15
584			min	-269.02	4	.357	15	-.008	3	0	1	0	1	-.007	4
585		8	max	-.377	10	.781	4	.068	4	0	3	0	5	-.002	15
586			min	-269.149	4	.238	15	-.008	3	0	1	0	1	-.007	4
587		9	max	-.317	10	.391	4	.049	4	0	3	0	5	-.002	15
588			min	-269.278	4	.119	15	-.008	3	0	1	0	1	-.008	4
589		10	max	-.257	10	0	1	.029	4	0	3	0	5	-.002	15
590			min	-269.406	4	0	1	-.008	3	0	1	0	1	-.008	4
591		11	max	-.197	10	-.119	15	.018	1	0	3	0	5	-.002	15
592			min	-269.535	4	-.391	4	-.008	3	0	1	0	1	-.008	4
593		12	max	-.137	10	-.238	15	.018	1	0	3	0	5	-.002	15
594			min	-269.664	4	-.781	4	-.014	5	0	1	0	1	-.007	4
595		13	max	-.077	10	-.357	15	.018	1	0	3	0	5	-.002	15
596			min	-269.792	4	-1.172	4	-.034	5	0	1	0	3	-.007	4
597		14	max	-.017	10	-.476	15	.018	1	0	3	0	4	-.002	15
598			min	-269.921	4	-1.562	4	-.054	5	0	1	0	3	-.006	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.043	10	-595	15	.018	1	0	3	0	4	-.002	15
600		min	-270.05	4	-1.953	4	-.074	5	0	1	0	3	-.005	4
601	16	max	.103	10	-.714	15	.018	1	0	3	0	4	-.001	15
602		min	-270.178	4	-2.344	4	-.094	5	0	1	0	3	-.004	4
603	17	max	.163	10	-.833	15	.018	1	0	3	0	1	0	15
604		min	-270.307	4	-2.734	4	-.113	5	0	1	0	3	-.003	4
605	18	max	.223	10	-.952	15	.018	1	0	3	0	1	0	15
606		min	-270.435	4	-3.125	4	-.133	5	0	1	0	5	-.002	4
607	19	max	.283	10	-1.071	15	.018	1	0	3	0	1	0	1
608		min	-270.564	4	-3.515	4	-.153	5	0	1	0	5	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.006	2	.014	1	2.01e-3	5	NC	3	NC	3	
2			min	-.003	3	-.005	3	-.02	5	-1.053e-3	1	4988.179	2	2210.585	1	
3		2	max	.003	1	.006	2	.013	1	2.038e-3	5	NC	3	NC	3	
4			min	-.002	3	-.004	3	-.019	5	-1.011e-3	1	5401.385	2	2396.361	1	
5		3	max	.003	1	.005	2	.012	1	2.067e-3	5	NC	3	NC	3	
6			min	-.002	3	-.004	3	-.018	5	-9.698e-4	1	5885.723	2	2614.915	1	
7		4	max	.003	1	.005	2	.01	1	2.095e-3	5	NC	3	NC	3	
8			min	-.002	3	-.004	3	-.017	5	-9.283e-4	1	6457.248	2	2874.23	1	
9		5	max	.003	1	.004	2	.009	1	2.123e-3	5	NC	3	NC	3	
10			min	-.002	3	-.004	3	-.016	5	-8.868e-4	1	7137.056	2	3184.965	1	
11		6	max	.002	1	.004	2	.008	1	2.151e-3	5	NC	1	NC	3	
12			min	-.002	3	-.004	3	-.015	5	-8.453e-4	1	7953.307	2	3561.598	1	
13		7	max	.002	1	.003	2	.007	1	2.179e-3	5	NC	1	NC	2	
14			min	-.002	3	-.004	3	-.014	5	-8.038e-4	1	8944.305	2	4024.219	1	
15		8	max	.002	1	.003	2	.007	1	2.207e-3	5	NC	1	NC	2	
16			min	-.002	3	-.003	3	-.013	5	-7.623e-4	1	NC	1	4601.38	1	
17		9	max	.002	1	.003	2	.006	1	2.235e-3	5	NC	1	NC	2	
18			min	-.001	3	-.003	3	-.012	5	-7.208e-4	1	NC	1	5334.811	1	
19		10	max	.002	1	.002	2	.005	1	2.263e-3	5	NC	1	NC	2	
20			min	-.001	3	-.003	3	-.011	5	-6.793e-4	1	NC	1	6287.544	1	
21		11	max	.001	1	.002	2	.004	1	2.291e-3	5	NC	1	NC	2	
22			min	-.001	3	-.003	3	-.01	5	-6.378e-4	1	NC	1	7558.64	1	
23		12	max	.001	1	.002	2	.003	1	2.319e-3	5	NC	1	NC	2	
24			min	-.001	3	-.002	3	-.009	5	-5.963e-4	1	NC	1	9311.485	1	
25		13	max	.001	1	.001	2	.003	1	2.347e-3	5	NC	1	NC	1	
26			min	0	3	-.002	3	-.008	5	-5.548e-4	1	NC	1	NC	1	
27		14	max	0	1	0	2	.002	1	2.375e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.006	5	-5.133e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	2.403e-3	5	NC	1	NC	1	
30			min	0	3	-.001	3	-.005	5	-4.718e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	2.431e-3	5	NC	1	NC	1	
32			min	0	3	-.001	3	-.004	5	-4.303e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	2.459e-3	5	NC	1	NC	1	
34			min	0	3	0	3	-.003	5	-3.888e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	2.488e-3	5	NC	1	NC	1	
36			min	0	3	0	3	-.001	5	-3.473e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	2.516e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-3.058e-4	1	NC	1	NC	1	
39		M3	1	max	0	1	0	1	0	1	1.39e-4	1	NC	1	NC	1
40				min	0	1	0	1	0	1	-1.145e-3	5	NC	1	NC	1
41	2		max	0	1	0	2	.006	5	1.773e-4	1	NC	1	NC	1	
42			min	0	10	0	3	0	1	-1.151e-3	5	NC	1	NC	1	



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	1	0	2	.012	5	2.156e-4	1	NC	1	NC	1
44			min	0	10	-.001	3	-.001	1	-1.157e-3	5	NC	1	7935.72	14
45		4	max	0	1	0	2	.018	5	2.538e-4	1	NC	1	NC	1
46			min	0	10	-.002	3	-.001	1	-1.163e-3	5	NC	1	5185.268	14
47		5	max	0	1	0	2	.025	4	2.921e-4	1	NC	1	NC	1
48			min	0	10	-.003	3	-.001	1	-1.168e-3	5	NC	1	3821.548	14
49		6	max	0	1	0	2	.031	4	3.303e-4	1	NC	1	NC	1
50			min	0	10	-.003	3	-.001	1	-1.174e-3	5	NC	1	3011.339	14
51		7	max	0	1	0	2	.037	4	3.686e-4	1	NC	1	NC	1
52			min	0	10	-.004	3	0	1	-1.18e-3	5	NC	1	2477.063	14
53		8	max	0	1	.001	2	.043	4	4.068e-4	1	NC	1	NC	1
54			min	0	10	-.005	3	0	1	-1.186e-3	5	NC	1	2099.869	14
55		9	max	0	1	.002	2	.05	4	4.451e-4	1	NC	1	NC	1
56			min	0	10	-.005	3	0	2	-1.192e-3	5	NC	1	1820.408	14
57		10	max	0	1	.002	2	.056	4	4.833e-4	1	NC	1	NC	1
58			min	0	10	-.005	3	0	10	-1.197e-3	5	NC	1	1605.759	14
59		11	max	0	1	.002	2	.062	4	5.216e-4	1	NC	1	NC	1
60			min	0	10	-.006	3	0	10	-1.203e-3	5	NC	1	1436.211	14
61		12	max	0	1	.003	2	.068	4	5.599e-4	1	NC	1	NC	1
62			min	0	10	-.006	3	0	12	-1.209e-3	5	NC	1	1299.245	14
63		13	max	0	1	.004	1	.074	4	5.981e-4	1	NC	1	NC	1
64			min	0	10	-.006	3	0	12	-1.215e-3	5	NC	1	1186.536	14
65		14	max	.001	1	.005	1	.08	4	6.364e-4	1	NC	3	NC	1
66			min	0	10	-.007	3	0	12	-1.221e-3	5	9841.508	1	1092.335	14
67		15	max	.001	1	.006	1	.086	4	6.746e-4	1	NC	3	NC	1
68			min	0	10	-.007	3	0	12	-1.226e-3	5	8221.128	1	1012.546	14
69		16	max	.001	1	.007	1	.091	4	7.129e-4	1	NC	3	NC	2
70			min	0	10	-.007	3	0	12	-1.232e-3	5	6981.609	1	944.17	14
71		17	max	.001	1	.008	1	.097	4	7.511e-4	1	NC	3	NC	2
72			min	0	10	-.007	3	0	12	-1.238e-3	5	6020.409	1	884.965	14
73		18	max	.001	1	.009	1	.103	4	7.894e-4	1	NC	3	NC	2
74			min	0	10	-.007	3	0	12	-1.244e-3	5	5266.44	1	833.218	14
75		19	max	.001	1	.01	1	.109	4	8.276e-4	1	NC	3	NC	2
76			min	0	10	-.007	3	0	12	-1.25e-3	5	4669.686	1	787.599	14
77	M4	1	max	.003	1	.007	2	0	12	4.732e-3	5	NC	1	NC	2
78			min	0	3	-.005	3	-.115	4	-9.288e-4	1	NC	1	168.11	4
79		2	max	.003	1	.007	2	0	12	4.732e-3	5	NC	1	NC	2
80			min	0	3	-.005	3	-.105	4	-9.288e-4	1	NC	1	183.268	4
81		3	max	.003	1	.006	2	0	12	4.732e-3	5	NC	1	NC	2
82			min	0	3	-.004	3	-.096	4	-9.288e-4	1	NC	1	201.31	4
83		4	max	.002	1	.006	2	0	12	4.732e-3	5	NC	1	NC	2
84			min	0	3	-.004	3	-.087	4	-9.288e-4	1	NC	1	222.996	4
85		5	max	.002	1	.006	2	0	12	4.732e-3	5	NC	1	NC	2
86			min	0	3	-.004	3	-.078	4	-9.288e-4	1	NC	1	249.361	4
87		6	max	.002	1	.005	2	0	12	4.732e-3	5	NC	1	NC	2
88			min	0	3	-.004	3	-.069	4	-9.288e-4	1	NC	1	281.845	4
89		7	max	.002	1	.005	2	0	12	4.732e-3	5	NC	1	NC	2
90			min	0	3	-.003	3	-.06	4	-9.288e-4	1	NC	1	322.497	4
91		8	max	.002	1	.004	2	0	12	4.732e-3	5	NC	1	NC	2
92			min	0	3	-.003	3	-.052	4	-9.288e-4	1	NC	1	374.32	4
93		9	max	.002	1	.004	2	0	12	4.732e-3	5	NC	1	NC	2
94			min	0	3	-.003	3	-.044	4	-9.288e-4	1	NC	1	441.853	4
95		10	max	.001	1	.004	2	0	12	4.732e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.036	4	-9.288e-4	1	NC	1	532.235	4
97		11	max	.001	1	.003	2	0	12	4.732e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.029	4	-9.288e-4	1	NC	1	657.221	4
99		12	max	.001	1	.003	2	0	12	4.732e-3	5	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	9	.003	1	.013	4	1.39e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	2	-1.173e-3	4	NC	1	NC	1
159		4	max	0	9	.004	1	.019	4	1.421e-5	1	NC	1	NC	1
160			min	0	2	-.004	3	0	2	-1.158e-3	4	NC	1	NC	1
161		5	max	0	9	.005	1	.026	4	1.451e-5	1	NC	3	NC	1
162			min	0	2	-.005	3	0	2	-1.143e-3	4	8893.567	1	NC	1
163		6	max	0	9	.007	1	.032	4	1.481e-5	1	NC	3	NC	1
164			min	0	2	-.007	3	0	1	-1.128e-3	4	7039.404	1	NC	1
165		7	max	0	9	.008	1	.039	4	1.512e-5	1	NC	3	NC	1
166			min	0	2	-.008	3	0	1	-1.113e-3	4	5777.992	1	NC	1
167		8	max	0	9	.009	1	.045	4	2.354e-5	3	NC	3	NC	1
168			min	0	2	-.009	3	0	1	-1.098e-3	4	4859.964	1	NC	1
169		9	max	0	9	.011	1	.051	4	3.202e-5	3	NC	3	NC	1
170			min	0	2	-.011	3	0	1	-1.084e-3	4	4160.559	1	NC	1
171		10	max	.001	9	.013	1	.058	4	4.05e-5	3	NC	3	NC	1
172			min	0	2	-.012	3	0	1	-1.069e-3	4	3610.218	1	NC	1
173		11	max	.001	9	.015	1	.064	4	4.897e-5	3	NC	3	NC	1
174			min	0	2	-.013	3	-.001	1	-1.054e-3	4	3166.881	1	NC	1
175		12	max	.001	9	.016	1	.07	4	5.745e-5	3	NC	3	NC	1
176			min	-.001	2	-.014	3	-.001	1	-1.039e-3	4	2803.441	1	NC	1
177		13	max	.001	9	.018	1	.076	4	6.593e-5	3	NC	3	NC	1
178			min	-.001	2	-.015	3	-.001	1	-1.024e-3	4	2501.514	1	NC	1
179		14	max	.001	9	.02	1	.082	4	7.441e-5	3	NC	3	NC	1
180			min	-.001	2	-.015	3	-.001	1	-1.009e-3	4	2248.098	1	NC	1
181		15	max	.002	9	.023	1	.088	4	8.288e-5	3	NC	3	NC	1
182			min	-.001	2	-.016	3	-.002	1	-9.943e-4	4	2033.68	1	NC	1
183		16	max	.002	9	.025	1	.093	4	9.136e-5	3	NC	3	NC	1
184			min	-.001	2	-.017	3	-.002	1	-9.794e-4	4	1851.11	1	NC	1
185		17	max	.002	9	.027	1	.099	4	9.984e-5	3	NC	3	NC	1
186			min	-.002	2	-.018	3	-.002	1	-9.645e-4	4	1694.891	1	NC	1
187		18	max	.002	9	.029	1	.105	4	1.083e-4	3	NC	3	NC	1
188			min	-.002	2	-.018	3	-.002	1	-9.497e-4	4	1560.727	1	NC	1
189		19	max	.002	9	.032	1	.11	4	1.168e-4	3	NC	3	NC	1
190			min	-.002	2	-.019	3	-.002	1	-9.348e-4	4	1445.214	1	NC	1
191	M8	1	max	.008	1	.023	2	.002	1	4.468e-3	4	NC	1	NC	2
192			min	-.002	3	-.014	3	-.116	4	-1.095e-4	1	NC	1	166.63	4
193		2	max	.008	1	.021	2	.002	1	4.468e-3	4	NC	1	NC	2
194			min	-.002	3	-.013	3	-.106	4	-1.095e-4	1	NC	1	181.654	4
195		3	max	.008	1	.02	2	.002	1	4.468e-3	4	NC	1	NC	1
196			min	-.002	3	-.013	3	-.097	4	-1.095e-4	1	NC	1	199.536	4
197		4	max	.007	1	.019	2	.002	1	4.468e-3	4	NC	1	NC	1
198			min	-.001	3	-.012	3	-.087	4	-1.095e-4	1	NC	1	221.031	4
199		5	max	.007	1	.018	2	.002	1	4.468e-3	4	NC	1	NC	1
200			min	-.001	3	-.011	3	-.078	4	-1.095e-4	1	NC	1	247.163	4
201		6	max	.006	1	.016	2	.001	1	4.468e-3	4	NC	1	NC	1
202			min	-.001	3	-.01	3	-.069	4	-1.095e-4	1	NC	1	279.36	4
203		7	max	.006	1	.015	2	.001	1	4.468e-3	4	NC	1	NC	1
204			min	-.001	3	-.01	3	-.06	4	-1.095e-4	1	NC	1	319.653	4
205		8	max	.005	1	.014	2	.001	1	4.468e-3	4	NC	1	NC	1
206			min	-.001	3	-.009	3	-.052	4	-1.095e-4	1	NC	1	371.018	4
207		9	max	.005	1	.013	2	0	1	4.468e-3	4	NC	1	NC	1
208			min	0	3	-.008	3	-.044	4	-1.095e-4	1	NC	1	437.955	4
209		10	max	.004	1	.011	2	0	1	4.468e-3	4	NC	1	NC	1
210			min	0	3	-.007	3	-.037	4	-1.095e-4	1	NC	1	527.539	4
211		11	max	.004	1	.01	2	0	1	4.468e-3	4	NC	1	NC	1
212			min	0	3	-.006	3	-.03	4	-1.095e-4	1	NC	1	651.421	4
213		12	max	.003	1	.009	2	0	1	4.468e-3	4	NC	1	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.021	5	7.424e-5	10	NC	1	909.077	5
329		13	max	0	1	.002	2	.002	1	6.101e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.016	5	7.424e-5	10	NC	1	1206.173	5
331		14	max	0	1	.002	2	.001	1	6.101e-3	4	NC	1	NC	1
332			min	0	3	-.001	3	-.011	5	7.424e-5	10	NC	1	1690.98	5
333		15	max	0	1	.002	2	0	1	6.101e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.008	5	7.424e-5	10	NC	1	2565.85	5
335		16	max	0	1	.001	2	0	1	6.101e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.004	5	7.424e-5	10	NC	1	4406.029	5
337		17	max	0	1	0	2	0	1	6.101e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.002	5	7.424e-5	10	NC	1	9445.664	5
339		18	max	0	1	0	2	0	1	6.101e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	7.424e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	6.101e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	7.424e-5	10	NC	1	NC	1
343	M1	1	max	.005	3	.021	3	.011	5	2.156e-2	1	NC	1	NC	1
344			min	-.006	2	-.032	1	-.004	1	-1.635e-2	3	NC	1	NC	1
345		2	max	.005	3	.011	3	.015	5	1.038e-2	1	NC	4	NC	2
346			min	-.006	2	-.017	1	-.01	1	-8.093e-3	3	3104.714	1	8288.182	1
347		3	max	.005	3	.002	3	.02	5	5.153e-4	5	NC	5	NC	2
348			min	-.006	2	-.003	1	-.014	1	-5.917e-4	1	1605.999	1	4867.357	5
349		4	max	.005	3	.009	1	.026	5	5.112e-4	5	NC	5	NC	2
350			min	-.006	2	-.005	3	-.016	1	-4.857e-4	1	1136.684	1	3094.679	5
351		5	max	.005	3	.019	1	.032	5	5.072e-4	5	NC	5	NC	2
352			min	-.006	2	-.012	3	-.016	1	-3.798e-4	1	911.244	1	2227.264	5
353		6	max	.005	3	.027	1	.038	5	5.031e-4	5	NC	5	NC	2
354			min	-.006	2	-.016	3	-.015	1	-2.738e-4	1	783.948	1	1718.89	5
355		7	max	.005	3	.034	1	.045	5	4.991e-4	5	NC	5	NC	2
356			min	-.006	2	-.02	3	-.013	1	-1.679e-4	1	707.012	1	1388.339	5
357		8	max	.005	3	.038	1	.051	5	4.95e-4	5	NC	5	NC	2
358			min	-.006	2	-.023	3	-.011	1	-6.195e-5	1	660.573	1	1158.298	5
359		9	max	.005	3	.041	1	.058	5	4.91e-4	5	NC	5	NC	1
360			min	-.006	2	-.024	3	-.008	1	5.242e-6	10	635.366	1	982.161	4
361		10	max	.005	3	.042	1	.065	5	5.077e-4	4	NC	5	NC	1
362			min	-.006	2	-.024	3	-.004	1	1.355e-5	10	627.133	1	844.927	4
363		11	max	.005	3	.041	1	.073	4	5.264e-4	4	NC	5	NC	1
364			min	-.006	2	-.023	3	-.001	1	2.185e-5	10	634.596	1	740.742	4
365		12	max	.005	3	.038	1	.08	4	5.45e-4	4	NC	5	NC	2
366			min	-.006	2	-.021	3	0	10	2.551e-5	12	658.936	1	659.895	4
367		13	max	.005	3	.033	1	.088	4	5.636e-4	4	NC	5	NC	2
368			min	-.006	2	-.019	3	0	12	2.755e-5	12	704.282	1	596.097	4
369		14	max	.005	3	.027	1	.095	4	5.823e-4	4	NC	5	NC	2
370			min	-.006	2	-.015	3	0	12	2.96e-5	12	779.661	1	545.129	4
371		15	max	.005	3	.018	1	.102	4	6.797e-4	1	NC	5	NC	2
372			min	-.006	2	-.01	3	0	12	3.164e-5	12	904.406	1	504.073	4
373		16	max	.005	3	.008	1	.109	4	9.655e-4	4	NC	5	NC	2
374			min	-.006	2	-.005	3	0	12	3.296e-5	12	1124.594	1	470.868	4
375		17	max	.005	3	.002	3	.115	4	9.562e-3	4	NC	5	NC	2
376			min	-.006	2	-.004	1	0	12	1.691e-5	12	1578.485	1	444.055	4
377		18	max	.005	3	.009	3	.12	4	1.2e-2	1	NC	4	NC	2
378			min	-.006	2	-.019	1	0	10	-3.838e-3	3	3042.672	1	422.515	4
379		19	max	.005	3	.016	3	.125	4	2.407e-2	1	NC	1	NC	1
380			min	-.006	2	-.034	1	-.003	1	-7.775e-3	3	NC	1	405.917	4
381	M5	1	max	.013	3	.064	3	.01	5	3.887e-6	4	NC	1	NC	1
382			min	-.02	2	-.095	1	-.005	1	5.587e-8	10	NC	1	NC	1
383		2	max	.013	3	.035	3	.015	5	2.438e-4	5	NC	5	NC	1
384			min	-.02	2	-.051	1	-.005	1	-9.635e-5	1	1044.076	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.013	3	.008	3	.02	5	4.798e-4	5	NC	5	NC	1
386			min	-.02	2	-.009	1	-.004	1	-1.911e-4	1	537.747	1	NC	1
387		4	max	.013	3	.026	1	.026	5	4.977e-4	5	NC	5	NC	1
388			min	-.02	2	-.015	3	-.003	1	-1.78e-4	1	379.816	1	NC	1
389		5	max	.013	3	.057	1	.033	5	5.155e-4	5	NC	15	NC	1
390			min	-.02	2	-.033	3	-.003	1	-1.65e-4	1	303.936	1	NC	1
391		6	max	.013	3	.082	1	.039	5	5.334e-4	5	NC	15	NC	1
392			min	-.02	2	-.047	3	-.003	1	-1.519e-4	1	261.039	1	NC	1
393		7	max	.013	3	.101	1	.047	5	5.513e-4	5	NC	15	NC	1
394			min	-.02	2	-.058	3	-.002	1	-1.389e-4	1	235.049	1	NC	1
395		8	max	.013	3	.115	1	.054	5	5.692e-4	5	9746.004	15	NC	1
396			min	-.02	2	-.065	3	-.002	1	-1.258e-4	1	219.282	1	NC	1
397		9	max	.013	3	.123	1	.061	5	5.871e-4	5	9399.27	15	NC	1
398			min	-.02	2	-.069	3	-.002	1	-1.128e-4	1	210.618	1	NC	1
399		10	max	.013	3	.126	1	.069	5	6.05e-4	5	9301.25	15	NC	1
400			min	-.02	2	-.069	3	-.002	1	-9.975e-5	1	207.614	1	NC	1
401		11	max	.013	3	.123	1	.076	4	6.229e-4	5	9434.915	15	NC	1
402			min	-.02	2	-.067	3	-.002	1	-8.67e-5	1	209.83	1	NC	1
403		12	max	.013	3	.115	1	.084	4	6.408e-4	5	9819.518	15	NC	1
404			min	-.02	2	-.061	3	-.002	1	-7.365e-5	1	217.642	1	NC	1
405		13	max	.013	3	.101	1	.091	4	6.587e-4	5	NC	15	NC	1
406			min	-.02	2	-.053	3	-.002	1	-6.06e-5	1	232.408	1	NC	1
407		14	max	.013	3	.081	1	.098	4	6.766e-4	5	NC	15	NC	1
408			min	-.02	2	-.042	3	-.002	1	-4.756e-5	1	257.114	1	9310.462	4
409		15	max	.013	3	.055	1	.104	4	6.945e-4	5	NC	15	NC	1
410			min	-.02	2	-.029	3	-.002	1	-3.451e-5	1	298.18	1	9156.116	4
411		16	max	.013	3	.024	1	.11	4	1.053e-3	5	NC	5	NC	1
412			min	-.02	2	-.013	3	-.002	1	-3.111e-5	2	370.995	1	9868.907	4
413		17	max	.013	3	.005	3	.116	4	9.599e-3	4	NC	5	NC	1
414			min	-.02	2	-.013	1	-.002	1	-1.94e-4	1	522.446	1	NC	1
415		18	max	.013	3	.025	3	.121	4	4.925e-3	4	NC	5	NC	1
416			min	-.02	2	-.057	1	-.002	1	-9.947e-5	1	1011.93	1	NC	1
417		19	max	.013	3	.046	3	.125	4	1.437e-6	5	NC	1	NC	1
418			min	-.02	2	-.104	1	-.003	1	-7.878e-8	3	NC	1	NC	1
419	M9	1	max	.005	3	.021	3	.008	5	1.635e-2	3	NC	1	NC	1
420			min	-.006	2	-.032	1	-.006	1	-2.156e-2	1	NC	1	NC	1
421		2	max	.005	3	.011	3	.008	5	8.111e-3	3	NC	4	NC	2
422			min	-.006	2	-.017	1	-.001	1	-1.065e-2	1	3105.543	1	9770.334	1
423		3	max	.005	3	.002	3	.008	4	4.98e-5	1	NC	5	NC	2
424			min	-.006	2	-.003	1	0	3	7.794e-6	10	1606.44	1	6085.253	1
425		4	max	.005	3	.009	1	.01	4	1.85e-5	5	NC	5	NC	2
426			min	-.006	2	-.005	3	0	3	-3.872e-5	1	1136.997	1	5171.686	1
427		5	max	.005	3	.019	1	.014	4	1.556e-6	3	NC	5	NC	2
428			min	-.006	2	-.012	3	0	3	-1.272e-4	1	911.488	1	5145.839	1
429		6	max	.005	3	.027	1	.018	4	-4.887e-6	12	NC	5	NC	2
430			min	-.006	2	-.017	3	-.001	3	-2.158e-4	1	784.148	1	4158.234	4
431		7	max	.005	3	.034	1	.023	4	-1.035e-5	12	NC	5	NC	2
432			min	-.006	2	-.02	3	-.001	3	-3.043e-4	1	707.182	1	2878.918	4
433		8	max	.005	3	.038	1	.029	4	-1.582e-5	12	NC	5	NC	1
434			min	-.006	2	-.023	3	-.002	3	-3.928e-4	1	660.722	1	2122.622	4
435		9	max	.005	3	.041	1	.035	4	-2.128e-5	12	NC	5	NC	1
436			min	-.006	2	-.024	3	-.004	1	-4.813e-4	1	635.5	1	1638.68	4
437		10	max	.005	3	.042	1	.043	5	-2.675e-5	12	NC	5	NC	1
438			min	-.006	2	-.024	3	-.006	1	-5.698e-4	1	627.255	1	1310.207	4
439		11	max	.005	3	.041	1	.051	5	-3.221e-5	12	NC	5	NC	2
440			min	-.006	2	-.023	3	-.009	1	-6.584e-4	1	634.709	1	1076.895	4
441		12	max	.005	3	.038	1	.059	5	-3.768e-5	12	NC	5	NC	2







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.002	1	.171	3	.121	1	8.516e-3	1	NC	5	NC	3
500		min	-.125	4	-.513	1	.006	10	-3.953e-3	3	426.351	1	1610.729	1
501	4	max	.002	1	.215	3	.183	1	9.82e-3	1	NC	15	NC	3
502		min	-.125	4	-.647	1	.011	10	-4.522e-3	3	332.789	1	1084.577	1
503	5	max	.002	1	.228	3	.213	1	1.112e-2	1	NC	15	NC	3
504		min	-.125	4	-.686	1	.012	10	-5.09e-3	3	313.084	1	934.804	1
505	6	max	.002	1	.212	3	.203	1	1.243e-2	1	NC	5	NC	3
506		min	-.125	4	-.63	1	.01	10	-5.659e-3	3	342.248	1	978.823	1
507	7	max	.002	1	.171	3	.156	1	1.373e-2	1	NC	5	NC	3
508		min	-.125	4	-.499	1	.006	10	-6.227e-3	3	438.317	1	1266.14	1
509	8	max	.002	1	.117	3	.085	1	1.504e-2	1	NC	5	NC	3
510		min	-.125	4	-.33	1	-.002	5	-6.796e-3	3	689.687	1	2268.44	1
511	9	max	.003	1	.068	3	.015	3	1.634e-2	1	NC	5	NC	1
512		min	-.125	4	-.174	1	-.008	5	-7.364e-3	3	1452.478	1	NC	1
513	10	max	.003	1	.046	3	.013	3	1.764e-2	1	NC	4	NC	1
514		min	-.125	4	-.104	1	-.02	2	-7.933e-3	3	2921.211	1	NC	1
515	11	max	.003	1	.068	3	.016	14	1.634e-2	1	NC	5	NC	1
516		min	-.125	4	-.175	1	-.007	10	-7.364e-3	3	1452.478	1	NC	1
517	12	max	.003	1	.117	3	.082	1	1.504e-2	1	NC	5	NC	3
518		min	-.125	4	-.33	1	-.001	10	-6.795e-3	3	689.687	1	2338.358	1
519	13	max	.003	1	.171	3	.152	1	1.373e-2	1	NC	5	NC	3
520		min	-.125	4	-.499	1	.006	10	-6.226e-3	3	438.317	1	1296.074	1
521	14	max	.003	1	.212	3	.199	1	1.243e-2	1	NC	5	NC	3
522		min	-.125	4	-.63	1	.003	15	-5.657e-3	3	342.248	1	1000.07	1
523	15	max	.003	1	.228	3	.208	1	1.113e-2	1	NC	15	NC	3
524		min	-.125	4	-.686	1	-.004	5	-5.089e-3	3	313.084	1	955.475	1
525	16	max	.003	1	.215	3	.178	1	9.822e-3	1	NC	15	NC	3
526		min	-.125	4	-.647	1	-.013	5	-4.52e-3	3	332.789	1	1111.239	1
527	17	max	.003	1	.171	3	.118	1	8.519e-3	1	NC	5	NC	3
528		min	-.125	4	-.513	1	-.017	5	-3.951e-3	3	426.351	1	1659.408	1
529	18	max	.003	1	.101	3	.046	1	7.215e-3	1	NC	5	NC	2
530		min	-.125	4	-.297	1	-.015	5	-3.382e-3	3	774.706	1	3967.152	1
531	19	max	.003	1	.016	3	.005	3	5.911e-3	1	NC	1	NC	1
532		min	-.125	4	-.034	1	-.006	2	-2.813e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	2.738e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.648e-4	5	NC	1	NC	1
535	2	max	0	1	-.003	15	.013	4	7.566e-4	3	NC	5	NC	1
536		min	-.001	5	-.023	6	0	3	-8.764e-4	1	4513.402	6	8222.544	4
537	3	max	0	1	-.006	15	.027	4	1.239e-3	3	NC	5	NC	1
538		min	-.002	5	-.046	6	-.003	3	-1.664e-3	1	2296.716	6	3861.482	4
539	4	max	0	1	-.008	15	.042	4	1.722e-3	3	NC	15	NC	2
540		min	-.003	5	-.067	6	-.006	3	-2.452e-3	1	1575.682	6	2517.062	4
541	5	max	0	1	-.011	15	.055	4	2.205e-3	3	9335.674	15	NC	9
542		min	-.004	5	-.086	6	-.009	3	-3.24e-3	1	1229.52	6	1907.489	4
543	6	max	0	1	-.013	15	.067	4	2.687e-3	3	7856.956	15	NC	10
544		min	-.005	5	-.102	6	-.013	3	-4.028e-3	1	1034.771	6	1587.412	4
545	7	max	0	1	-.014	15	.075	4	3.17e-3	3	6967.703	15	8988.578	10
546		min	-.007	5	-.115	6	-.017	3	-4.816e-3	1	917.655	6	1414.528	4
547	8	max	0	1	-.015	15	.079	4	3.653e-3	3	6434.017	15	7436.998	10
548		min	-.008	5	-.124	6	-.021	3	-5.604e-3	1	847.368	6	1332.968	4
549	9	max	0	1	-.016	15	.08	4	4.136e-3	3	6146.756	15	6418.91	10
550		min	-.009	5	-.13	6	-.025	3	-6.392e-3	1	809.536	6	1320.769	4
551	10	max	0	1	-.016	15	.077	4	4.618e-3	3	6055.88	15	5746.262	10
552		min	-.01	5	-.132	6	-.028	3	-7.18e-3	1	797.567	6	1374.412	4
553	11	max	0	1	-.015	15	.07	4	5.101e-3	3	6146.756	15	5320.12	10
554		min	-.011	5	-.129	6	-.03	3	-7.968e-3	1	809.536	6	1506.401	4
555	12	max	0	1	-.014	15	.06	4	5.584e-3	3	6434.017	15	5091.606	10



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-.012	5	-.123	6	-.032	3	-8.756e-3	1	847.368	6	1752.537	4
557	13	max	0	1	-.013	15	.048	4	6.067e-3	3	6967.703	15	5047.254	10
558		min	-.013	5	-.114	1	-.031	3	-9.544e-3	1	917.655	6	1795.455	1
559	14	max	0	1	-.011	15	.043	1	6.549e-3	3	7856.956	15	6104.955	15
560		min	-.014	5	-.102	1	-.029	3	-1.033e-2	1	1034.771	6	1849	1
561	15	max	0	1	-.009	15	.037	1	7.032e-3	3	9335.674	15	NC	15
562		min	-.015	5	-.086	1	-.025	3	-1.112e-2	1	1229.52	6	2005.198	1
563	16	max	0	1	-.006	15	.028	1	7.515e-3	3	NC	15	NC	4
564		min	-.016	5	-.069	1	-.019	3	-1.191e-2	1	1575.682	6	2341.561	1
565	17	max	0	1	-.003	15	.016	1	7.997e-3	3	NC	5	NC	4
566		min	-.017	5	-.049	1	-.011	3	-1.27e-2	1	2296.716	6	3101.655	1
567	18	max	0	1	0	15	.002	9	8.48e-3	3	NC	5	NC	4
568		min	-.018	5	-.027	1	-.006	5	-1.348e-2	1	4513.402	6	5518.058	1
569	19	max	0	1	.005	5	.015	3	8.963e-3	3	NC	1	NC	1
570		min	-.02	5	-.005	1	-.021	2	-1.427e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	.005	3	3.022e-3	3	NC	1	NC	1
572		min	-.008	4	-.003	4	-.006	2	-4.482e-3	1	NC	1	NC	1
573	2	max	0	10	-.011	12	.006	1	2.884e-3	3	NC	15	NC	2
574		min	-.007	4	-.037	4	-.003	5	-4.261e-3	1	3105.132	4	9383.262	1
575	3	max	0	10	-.022	12	.015	1	2.745e-3	3	5185.08	15	NC	3
576		min	-.007	4	-.07	4	-.011	5	-4.04e-3	1	1580.096	4	5305.363	1
577	4	max	0	10	-.031	15	.022	1	2.606e-3	3	3557.268	15	NC	10
578		min	-.006	4	-.101	4	-.024	5	-3.819e-3	1	1084.038	4	4031.939	1
579	5	max	0	10	-.04	15	.026	1	2.467e-3	3	2775.772	15	NC	10
580		min	-.006	4	-.128	4	-.04	5	-3.597e-3	1	845.886	4	2734.411	5
581	6	max	0	10	-.047	15	.029	1	2.328e-3	3	2336.106	15	NC	10
582		min	-.005	4	-.152	4	-.056	5	-3.376e-3	1	711.902	4	1932.128	5
583	7	max	0	10	-.053	15	.03	1	2.19e-3	3	2071.704	15	NC	10
584		min	-.005	4	-.171	4	-.071	5	-3.155e-3	1	631.329	4	1511.042	5
585	8	max	0	10	-.057	15	.029	1	2.051e-3	3	1913.024	15	NC	10
586		min	-.005	4	-.184	4	-.084	5	-2.934e-3	1	582.973	4	1271.025	5
587	9	max	0	10	-.059	15	.028	1	1.912e-3	3	1827.612	15	NC	10
588		min	-.004	4	-.193	4	-.095	5	-2.713e-3	1	556.945	4	1132.147	5
589	10	max	0	10	-.06	15	.025	1	1.773e-3	3	1800.592	15	NC	10
590		min	-.004	4	-.195	4	-.101	5	-2.492e-3	1	548.711	4	1058.692	5
591	11	max	0	10	-.059	15	.022	1	1.634e-3	3	1827.612	15	NC	10
592		min	-.003	4	-.192	4	-.103	5	-2.271e-3	1	556.945	4	1034.886	5
593	12	max	0	10	-.056	15	.018	1	1.496e-3	3	1913.024	15	NC	3
594		min	-.003	4	-.184	4	-.101	5	-2.049e-3	1	582.973	4	1056.45	5
595	13	max	0	10	-.052	15	.014	1	1.357e-3	3	2071.704	15	NC	2
596		min	-.003	4	-.17	4	-.095	5	-1.828e-3	1	631.329	4	1128.757	5
597	14	max	0	10	-.046	15	.01	1	1.218e-3	3	2336.106	15	NC	2
598		min	-.002	4	-.15	4	-.084	5	-1.607e-3	1	711.902	4	1270.072	5
599	15	max	0	10	-.039	15	.007	1	1.079e-3	3	2775.772	15	NC	1
600		min	-.002	4	-.126	4	-.07	5	-1.386e-3	1	845.886	4	1523.797	5
601	16	max	0	10	-.03	15	.004	1	9.403e-4	3	3557.268	15	NC	1
602		min	-.001	4	-.099	4	-.053	5	-1.165e-3	1	1084.038	4	1998.116	5
603	17	max	0	10	-.021	15	.001	1	8.015e-4	3	5185.08	15	NC	1
604		min	0	4	-.068	4	-.035	5	-9.437e-4	1	1580.096	4	3027.69	5
605	18	max	0	10	-.011	15	0	9	7.319e-4	5	NC	15	NC	1
606		min	0	4	-.034	4	-.017	5	-7.226e-4	1	3105.132	4	6312.698	5
607	19	max	0	1	0	1	0	1	8.116e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.057e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5}$  (Eq. D-7)

k <sub>c</sub>	λ	f' <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b$  (Sec. D.4.1 & Eq. D-5)

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$N_{a0} = \tau_{k,cr} \pi d_a h_{ef}$  (Eq. D-16f)

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}$  (Sec. D.4.1 & Eq. D-16b)

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

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