

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.000	(Pressure)
$C_{f+ BOTTOM}$ =	1.600	
$C_{f- TOP, OUTER PURLIN}$ =	-2.300	
$C_{f- TOP, INNER PURLIN}$ =	-1.780	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.05	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

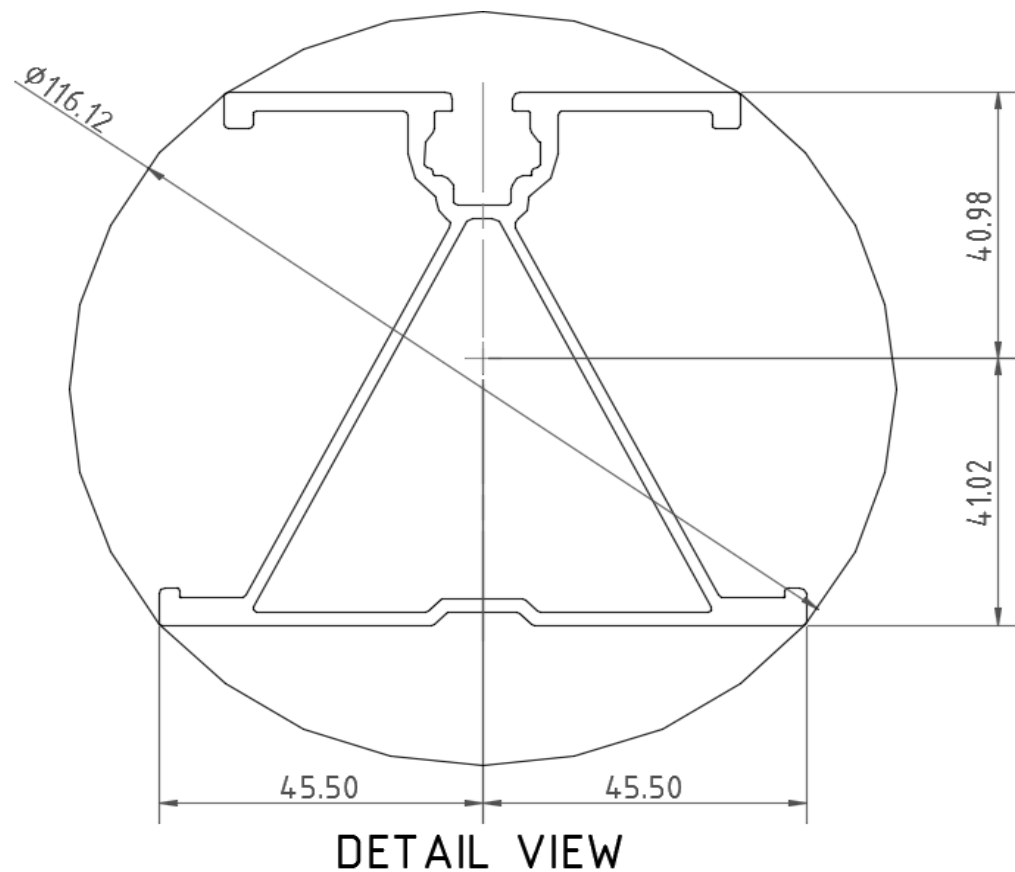
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	123 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	2.038 k-ft
$M_z$ =	0.267 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>96%</b>



### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

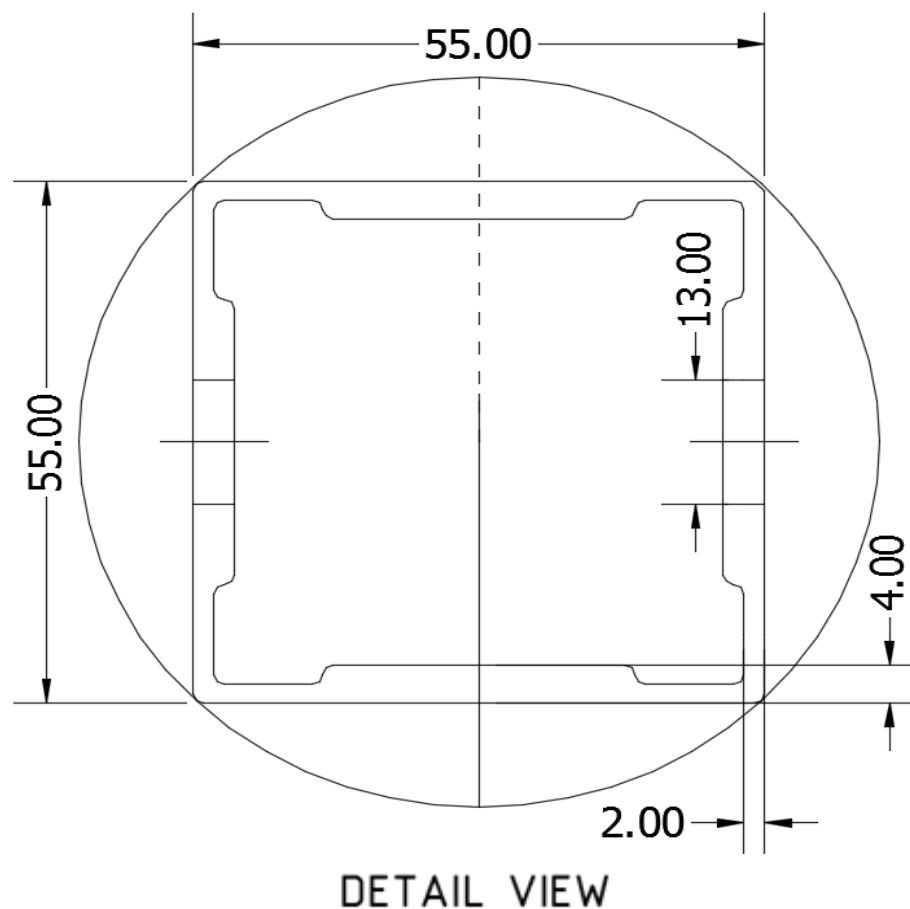
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.168 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.197 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>92%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

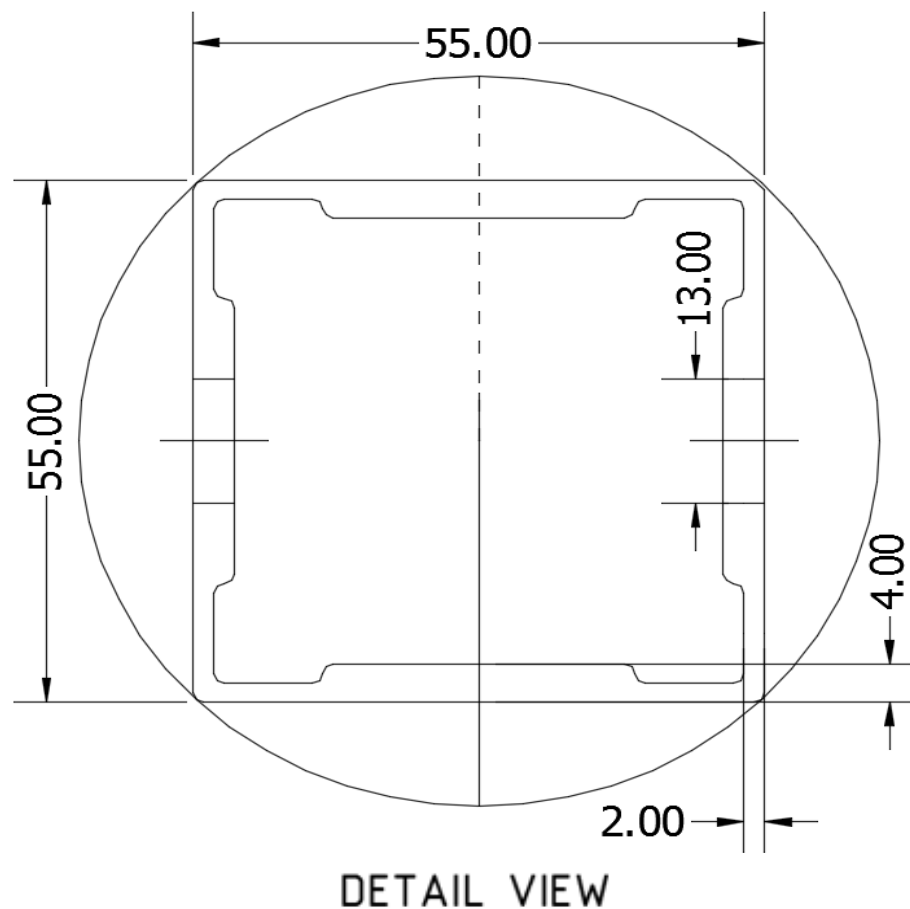
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>24.80</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.480 k-ft
$P_n$ =	0.704 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<u>37%</u>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	<u>86.60</u> in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.003 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<u>14%</u>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	48.30 in
$\Phi F_{ty \text{ AXIAL}}$ =	18.93 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	-0.335 k-ft
$P_n$ =	0.713 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	18.592 k
Utilization =	<b>28%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

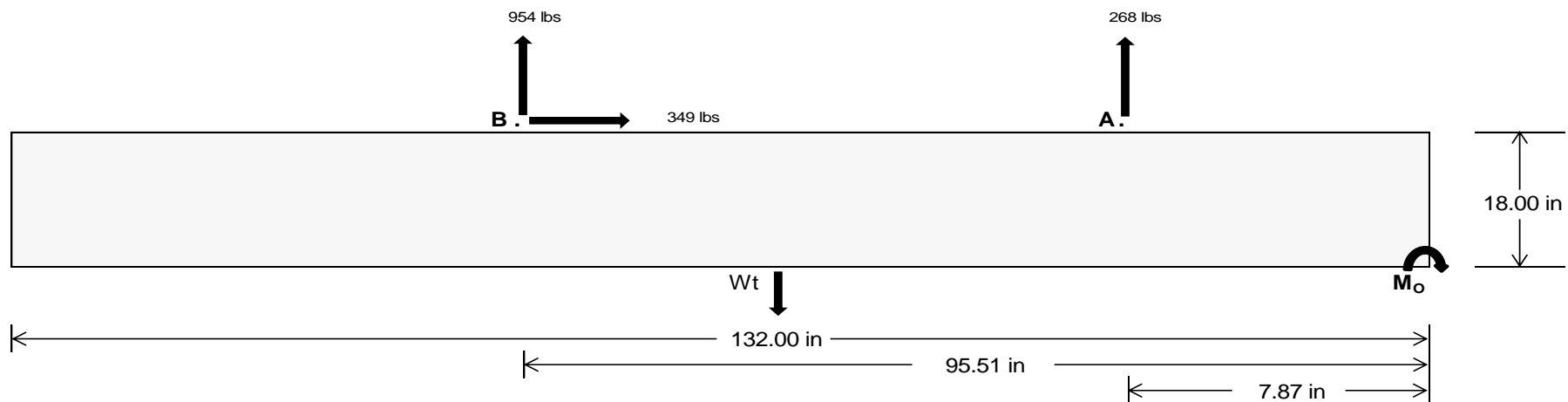
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>1127.32</b>	<b>3983.64</b> k
Compressive Load =	<b>4691.21</b>	<b>4868.46</b> k
Lateral Load =	<b>313.75</b>	<b>1454.67</b> k
Moment (Weak Axis) =	<b>0.64</b>	<b>0.43</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 99562.1$  in-lbs  
Resisting Force Required = 1508.52 lbs  
S.F. = 1.67  
Weight Required = 2514.19 lbs  
Minimum Width = 23 in in  
Weight Provided = 4585.63 lbs

### Sliding

Force = 349.48 lbs  
Friction = 0.4  
Weight Required = 873.71 lbs  
Resisting Weight = 4585.63 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 349.48 lbs  
Cohesion = 130 psf  
Area = 21.08 ft<sup>2</sup>  
Resisting = 2292.81 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Bearing Pressure

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 132in long x 23in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 23in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 23in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

	Ballast Width			
	23 in	24 in	25 in	26 in
$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(1.92 \text{ ft}) =$	<u>4586 lbs</u>	<u>4785 lbs</u>	<u>4984 lbs</u>	<u>5184 lbs</u>

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in	23 in	24 in	25 in	26 in
$F_A$	1767 lbs	1767 lbs	1767 lbs	1767 lbs	1372 lbs	1372 lbs	1372 lbs	1372 lbs	2223 lbs	2223 lbs	2223 lbs	2223 lbs	-536 lbs	-536 lbs	-536 lbs	-536 lbs
$F_B$	1831 lbs	1831 lbs	1831 lbs	1831 lbs	1423 lbs	1423 lbs	1423 lbs	1423 lbs	2305 lbs	2305 lbs	2305 lbs	2305 lbs	-1909 lbs	-1909 lbs	-1909 lbs	-1909 lbs
$F_V$	157 lbs	157 lbs	157 lbs	157 lbs	626 lbs	626 lbs	626 lbs	626 lbs	577 lbs	577 lbs	577 lbs	577 lbs	-699 lbs	-699 lbs	-699 lbs	-699 lbs
$P_{total}$	8184 lbs	8384 lbs	8583 lbs	8782 lbs	7380 lbs	7580 lbs	7779 lbs	7979 lbs	9114 lbs	9313 lbs	9513 lbs	9712 lbs	307 lbs	426 lbs	546 lbs	666 lbs
$M$	4292 lbs-ft	4292 lbs-ft	4292 lbs-ft	4292 lbs-ft	4084 lbs-ft	4084 lbs-ft	4084 lbs-ft	4084 lbs-ft	5964 lbs-ft	5964 lbs-ft	5964 lbs-ft	5964 lbs-ft	1051 lbs-ft	1051 lbs-ft	1051 lbs-ft	1051 lbs-ft
$e$	0.52 ft	0.51 ft	0.50 ft	0.49 ft	0.55 ft	0.54 ft	0.53 ft	0.51 ft	0.65 ft	0.64 ft	0.63 ft	0.61 ft	3.43 ft	2.47 ft	1.93 ft	1.58 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	277.2 psf	274.7 psf	272.4 psf	270.3 psf	244.4 psf	243.3 psf	242.2 psf	241.3 psf	278.0 psf	275.5 psf	273.1 psf	271.0 psf	0.0 psf	0.0 psf	0.0 psf	3.9 psf
$f_{max}$	499.2 psf	487.5 psf	476.7 psf	466.7 psf	455.7 psf	445.8 psf	436.7 psf	428.2 psf	586.6 psf	571.2 psf	557.1 psf	544.0 psf	51.5 psf	46.8 psf	48.9 psf	52.0 psf

Maximum Bearing Pressure = 587 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

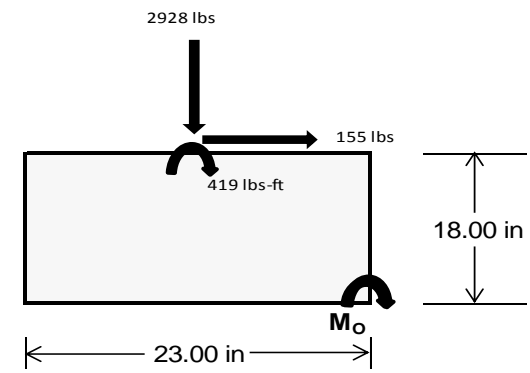
### Overturning Check

$M_o = 2154.2 \text{ ft-lbs}$   
 Resisting Force Required = 2247.90 lbs  
 S.F. = 1.67  
 Weight Required = 3746.50 lbs  
 Minimum Width = **23 in**  
 Weight Provided = 4585.63 lbs

*A minimum 132in long x 23in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	23 in			23 in			23 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_Y$	256 lbs	661 lbs	228 lbs	971 lbs	2928 lbs	949 lbs	85 lbs	193 lbs	57 lbs
$F_V$	216 lbs	213 lbs	218 lbs	161 lbs	155 lbs	169 lbs	216 lbs	214 lbs	217 lbs
$P_{\text{total}}$	5933 lbs	6338 lbs	5905 lbs	6375 lbs	8332 lbs	6353 lbs	1745 lbs	1853 lbs	1717 lbs
$M$	866 lbs-ft	859 lbs-ft	871 lbs-ft	657 lbs-ft	652 lbs-ft	681 lbs-ft	862 lbs-ft	856 lbs-ft	864 lbs-ft
$e$	0.15 ft	0.14 ft	0.15 ft	0.10 ft	0.08 ft	0.11 ft	0.49 ft	0.46 ft	0.50 ft
$L/6$	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft	0.32 ft
$f_{\text{min}}$	152.8 psf	173.0 psf	150.7 psf	204.8 psf	298.4 psf	200.2 psf	0.0 psf	0.0 psf	0.0 psf
$f_{\text{max}}$	410.1 psf	428.2 psf	409.4 psf	399.9 psf	491.9 psf	402.5 psf	227.9 psf	226.2 psf	228.7 psf



Maximum Bearing Pressure = 492 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 23in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 23in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

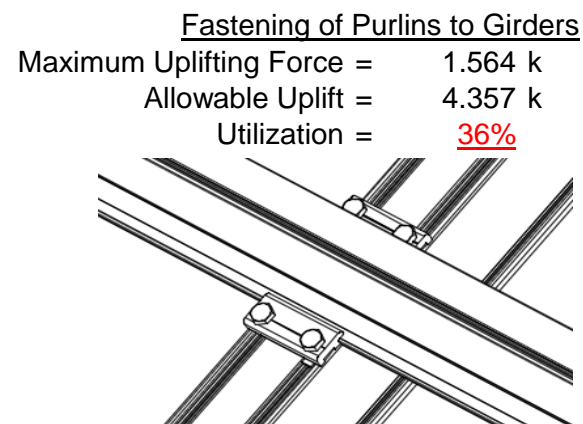
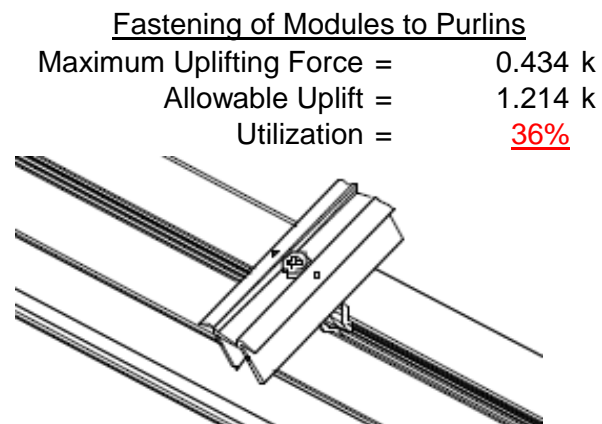
Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

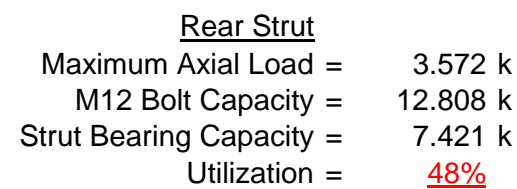
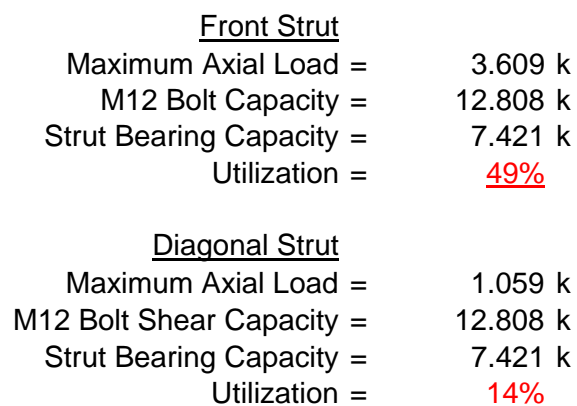
### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

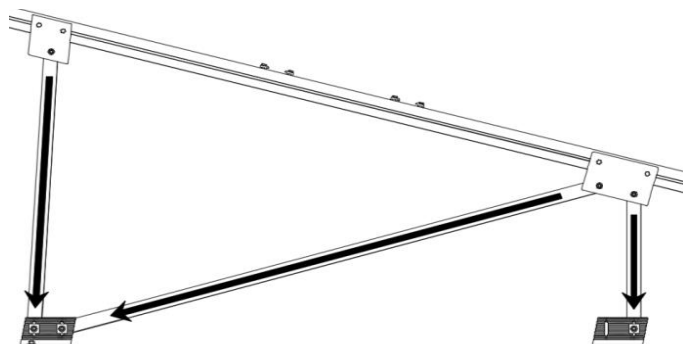


### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.



Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)

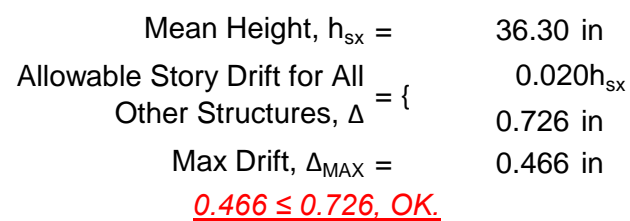


Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

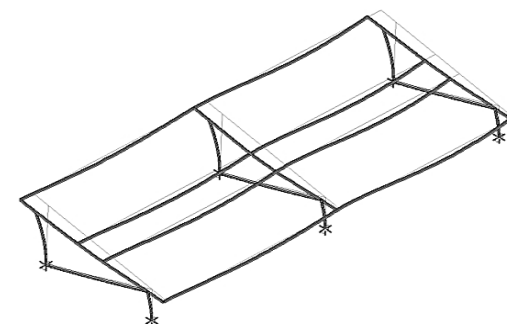
## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).



The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

**3.4.14**

$$L_b = 123 \text{ in}$$

$$J = 0.432$$

$$340.276$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 27.3 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 123$$

$$J = 0.432$$

$$216.395$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

**3.4.16**

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

**3.4.16**

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

### Strong Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

### Weak Axis:

#### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

#### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{\max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{\max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 31.6 \text{ ksi}$$
  

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$
  

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{\max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

**3.4.14**

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

**3.4.14**

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.4$$

**3.4.16**

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp^*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

#### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.30 \text{ in} \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 48.3 \\ J &= 0.942 \\ &= 75.3767 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.6 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.11734$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76536$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 18.9268 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 18.93 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 19.48 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-61.093	-61.093	0	0
2	M14	Y	-61.093	-61.093	0	0
3	M15	Y	-61.093	-61.093	0	0
4	M16	Y	-61.093	-61.093	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-35.466	-35.466	0	0
2	M14	y	-35.466	-35.466	0	0
3	M15	y	-56.746	-56.746	0	0
4	M16	y	-56.746	-56.746	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	81.572	81.572	0	0
2	M14	y	63.13	63.13	0	0
3	M15	y	35.466	35.466	0	0
4	M16	y	35.466	35.466	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19	10	max	88.776	1	865.204	1	-4.616	12	.005	14	.268	1	1.817	1
20		min	3.51	12	-803.078	3	-150.889	1	-.013	1	.007	12	-1.647	3
21	11	max	88.776	1	712.328	1	-3.563	12	.013	1	.114	1	.919	1
22		min	3.51	12	-660.33	3	-118.584	1	0	3	.002	12	-.814	3
23	12	max	88.776	1	559.452	1	-2.51	12	.013	1	.035	4	.194	1
24		min	3.51	12	-517.583	3	-86.28	1	0	3	-.003	1	-.143	3
25	13	max	88.776	1	406.577	1	-1.457	12	.013	1	.016	5	.365	3
26		min	3.51	12	-374.836	3	-53.975	1	0	3	-.082	1	-.356	1
27	14	max	88.776	1	253.701	1	-.404	12	.013	1	0	15	.711	3
28		min	.768	15	-232.089	3	-21.67	1	0	3	-.125	1	-.732	1
29	15	max	88.776	1	100.826	1	10.634	1	.013	1	-.004	12	.894	3
30		min	-10.431	5	-89.341	3	-12.477	5	0	3	-.132	1	-.934	1
31	16	max	88.776	1	53.406	3	42.939	1	.013	1	-.003	12	.914	3
32		min	-22.122	5	-52.05	1	-10.848	5	0	3	-.101	1	-.961	1
33	17	max	88.776	1	196.153	3	75.243	1	.013	1	0	12	.772	3
34		min	-33.813	5	-204.926	1	-9.219	5	0	3	-.049	4	-.815	1
35	18	max	88.776	1	338.9	3	107.548	1	.013	1	.07	1	.467	3
36		min	-45.504	5	-357.801	1	-7.59	5	0	3	-.051	5	-.495	1
37	19	max	88.776	1	481.648	3	139.852	1	.013	1	.211	1	0	1
38		min	-57.195	5	-510.677	1	-5.96	5	0	3	-.058	5	0	3
39	M14	1	max	63.36	4	538.389	1	-4.996	12	.006	.24	1	0	1
40		min	1.51	12	-378.414	3	-144.09	1	-.011	1	.009	12	0	3
41	2	max	51.669	4	385.514	1	-3.942	12	.006	3	.104	4	.369	3
42		min	1.51	12	-269.272	3	-111.786	1	-.011	1	.004	12	-.526	1
43	3	max	40.777	1	232.638	1	-2.889	12	.006	3	.056	5	.613	3
44		min	1.51	12	-160.13	3	-79.481	1	-.011	1	-.015	1	-.878	1
45	4	max	40.777	1	79.762	1	-1.836	12	.006	3	.03	5	.734	3
46		min	1.51	12	-50.989	3	-47.177	1	-.011	1	-.087	1	-1.056	1
47	5	max	40.777	1	58.153	3	-.783	12	.006	3	.006	5	.729	3
48		min	1.51	12	-73.113	1	-23.778	4	-.011	1	-.122	1	-1.06	1
49	6	max	40.777	1	167.295	3	17.432	1	.006	3	-.004	12	.601	3
50		min	-3.427	5	-225.989	1	-18.655	5	-.011	1	-.121	1	-.889	1
51	7	max	40.777	1	276.437	3	49.737	1	.006	3	-.003	12	.348	3
52		min	-15.118	5	-378.864	1	-17.026	5	-.011	1	-.082	1	-.545	1
53	8	max	40.777	1	385.579	3	82.041	1	.006	3	0	10	0	15
54		min	-26.808	5	-531.74	1	-15.397	5	-.011	1	-.058	4	-.032	2
55	9	max	40.777	1	494.721	3	114.346	1	.006	3	.104	1	.666	1
56		min	-38.499	5	-684.616	1	-13.767	5	-.011	1	-.072	5	-.53	3
57	10	max	62.173	4	837.491	1	-4.482	12	.006	3	.253	1	1.533	1
58		min	1.51	12	-603.862	3	-146.65	1	-.011	1	.007	12	-1.155	3
59	11	max	50.482	4	684.616	1	-3.429	12	.011	1	.105	4	.666	1
60		min	1.51	12	-494.721	3	-114.346	1	-.006	3	.002	12	-.53	3
61	12	max	40.777	1	531.74	1	-2.376	12	.011	1	.054	5	0	15
62		min	1.51	12	-385.579	3	-82.041	1	-.006	3	-.007	1	-.032	2
63	13	max	40.777	1	378.864	1	-1.323	12	.011	1	.028	5	.348	3
64		min	1.51	12	-276.437	3	-49.737	1	-.006	3	-.082	1	-.545	1
65	14	max	40.777	1	225.989	1	-.27	12	.011	1	.004	5	.601	3
66		min	1.51	12	-167.295	3	-24.335	4	-.006	3	-.121	1	-.889	1
67	15	max	40.777	1	73.113	1	14.872	1	.011	1	-.004	12	.729	3
68		min	-4.325	5	-58.153	3	-18.768	5	-.006	3	-.122	1	-1.06	1
69	16	max	40.777	1	50.989	3	47.177	1	.011	1	-.002	12	.734	3
70		min	-16.016	5	-79.762	1	-17.139	5	-.006	3	-.087	1	-1.056	1
71	17	max	40.777	1	160.13	3	79.481	1	.011	1	0	3	.613	3
72		min	-27.707	5	-232.638	1	-15.509	5	-.006	3	-.061	4	-.878	1
73	18	max	40.777	1	269.272	3	111.786	1	.011	1	.094	1	.369	3
74		min	-39.397	5	-385.514	1	-13.88	5	-.006	3	-.074	5	-.526	1
75	19	max	40.777	1	378.414	3	144.09	1	.011	1	.24	1	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76	M15	min	-51.088	5	-538.389	1	-12.251	5	-.006	3	-.089	5	0	3
77		max	80.277	5	603.816	1	-4.972	12	.011	1	.24	1	0	2
78		min	-42.856	1	-208.447	3	-144.07	1	-.005	3	.009	12	0	3
79		2 max	68.586	5	431.552	1	-3.919	12	.011	1	.14	4	.204	3
80		min	-42.856	1	-149.715	3	-111.766	1	-.005	3	.004	12	-.59	1
81		3 max	56.895	5	259.288	1	-2.866	12	.011	1	.081	5	.341	3
82		min	-42.856	1	-90.983	3	-79.461	1	-.005	3	-.015	1	-.983	1
83		4 max	45.204	5	87.024	1	-1.813	12	.011	1	.045	5	.411	3
84		min	-42.856	1	-32.251	3	-47.157	1	-.005	3	-.087	1	-1.18	1
85		5 max	33.513	5	26.481	3	-.76	12	.011	1	.011	5	.414	3
86		min	-42.856	1	-85.24	1	-32.517	4	-.005	3	-.122	1	-1.181	1
87		6 max	21.822	5	85.213	3	17.453	1	.011	1	-.004	12	.351	3
88		min	-42.856	1	-257.504	1	-27.392	5	-.005	3	-.121	1	-.986	1
89		7 max	10.131	5	143.945	3	49.757	1	.011	1	-.003	12	.22	3
90		min	-42.856	1	-429.768	1	-25.763	5	-.005	3	-.082	1	-.595	1
91		8 max	-1.007	15	202.677	3	82.062	1	.011	1	0	10	.023	3
92		min	-42.856	1	-602.032	1	-24.133	5	-.005	3	-.082	4	-.009	9
93		9 max	-1.731	12	261.409	3	114.366	1	.011	1	.104	1	.777	1
94		min	-42.856	1	-774.296	1	-22.504	5	-.005	3	-.106	5	-.241	3
95		10 max	-1.731	12	946.56	1	-4.505	12	.011	1	.253	1	1.757	1
96		min	-42.856	1	-320.14	3	-146.671	1	-.005	3	.007	12	-.572	3
97		11 max	8.747	5	774.296	1	-3.452	12	.005	3	.139	4	.777	1
98		min	-42.856	1	-261.409	3	-114.366	1	-.011	1	.002	12	-.241	3
99		12 max	-1.731	12	602.032	1	-2.399	12	.005	3	.079	5	.023	3
100		min	-42.856	1	-202.677	3	-82.062	1	-.011	1	-.007	1	-.009	9
101		13 max	-1.731	12	429.768	1	-1.346	12	.005	3	.043	5	.22	3
102		min	-42.856	1	-143.945	3	-49.757	1	-.011	1	-.082	1	-.595	1
103		14 max	-1.731	12	257.504	1	-.293	12	.005	3	.009	5	.351	3
104		min	-42.856	1	-85.213	3	-33.087	4	-.011	1	-.121	1	-.986	1
105		15 max	-1.731	12	85.24	1	14.852	1	.005	3	-.004	12	.414	3
106		min	-47.149	4	-26.481	3	-27.506	5	-.011	1	-.122	1	-1.181	1
107		16 max	-1.731	12	32.251	3	47.157	1	.005	3	-.002	12	.411	3
108		min	-58.84	4	-87.024	1	-25.876	5	-.011	1	-.087	1	-1.18	1
109		17 max	-1.731	12	90.983	3	79.461	1	.005	3	0	3	.341	3
110		min	-70.531	4	-259.288	1	-24.247	5	-.011	1	-.087	4	-.983	1
111		18 max	-1.731	12	149.715	3	111.766	1	.005	3	.094	1	.204	3
112		min	-82.222	4	-431.552	1	-22.618	5	-.011	1	-.109	5	-.59	1
113		19 max	-1.731	12	208.447	3	144.07	1	.005	3	.24	1	0	2
114		min	-93.913	4	-603.816	1	-20.989	5	-.011	1	-.134	5	0	5
115	M16	1 max	80.103	5	576.355	1	-4.786	12	.012	1	.212	1	0	1
116		min	-93.673	1	-195.379	3	-140.023	1	-.007	3	.008	12	0	3
117		2 max	68.412	5	404.091	1	-3.733	12	.012	1	.104	4	.189	3
118		min	-93.673	1	-136.647	3	-107.719	1	-.007	3	.003	12	-.558	1
119		3 max	56.721	5	231.827	1	-2.68	12	.012	1	.06	5	.311	3
120		min	-93.673	1	-77.916	3	-75.414	1	-.007	3	-.033	1	-.92	1
121		4 max	45.03	5	59.563	1	-1.627	12	.012	1	.033	5	.367	3
122		min	-93.673	1	-19.184	3	-43.11	1	-.007	3	-.101	1	-1.086	1
123		5 max	33.339	5	39.548	3	-.574	12	.012	1	.008	5	.355	3
124		min	-93.673	1	-112.701	1	-23.357	4	-.007	3	-.131	1	-1.056	1
125		6 max	21.648	5	98.28	3	21.499	1	.012	1	-.004	12	.276	3
126		min	-93.673	1	-284.965	1	-19.185	5	-.007	3	-.125	1	-.83	1
127		7 max	9.958	5	157.012	3	53.804	1	.012	1	-.003	12	.131	3
128		min	-93.673	1	-457.229	1	-17.556	5	-.007	3	-.083	1	-.407	1
129		8 max	-1.1	15	215.744	3	86.108	1	.012	1	0	10	.212	1
130		min	-93.673	1	-629.493	1	-15.927	5	-.007	3	-.056	4	-.081	3
131		9 max	-3.559	12	274.476	3	118.413	1	.012	1	.114	1	1.027	1
132		min	-93.673	1	-801.757	1	-14.297	5	-.007	3	-.072	5	-.36	3





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133		10	max	-3.559	12	974.021	1	-4.691	12	.012	1	.267	1	2.038	1
134			min	-93.673	1	-333.208	3	-150.717	1	-.007	3	.008	12	-.706	3
135		11	max	4.006	5	801.757	1	-3.638	12	.007	3	.114	1	1.027	1
136			min	-93.673	1	-274.476	3	-118.413	1	-.012	1	.003	12	-.36	3
137		12	max	-3.559	12	629.493	1	-2.585	12	.007	3	.054	4	.212	1
138			min	-93.673	1	-215.744	3	-86.108	1	-.012	1	-.003	1	-.081	3
139		13	max	-3.559	12	457.229	1	-1.532	12	.007	3	.027	5	.131	3
140			min	-93.673	1	-157.012	3	-53.804	1	-.012	1	-.083	1	-.407	1
141		14	max	-3.559	12	284.965	1	-.479	12	.007	3	.002	5	.276	3
142			min	-93.673	1	-98.28	3	-25.97	4	-.012	1	-.125	1	-.83	1
143		15	max	-3.559	12	112.701	1	10.805	1	.007	3	-.004	12	.355	3
144			min	-93.673	1	-39.548	3	-19.715	5	-.012	1	-.131	1	-1.056	1
145		16	max	-3.559	12	19.184	3	43.11	1	.007	3	-.003	12	.367	3
146			min	-93.673	1	-59.563	1	-18.086	5	-.012	1	-.101	1	-1.086	1
147		17	max	-3.559	12	77.916	3	75.414	1	.007	3	0	12	.311	3
148			min	-93.673	1	-231.827	1	-16.457	5	-.012	1	-.071	4	-.92	1
149		18	max	-3.559	12	136.647	3	107.719	1	.007	3	.071	1	.189	3
150			min	-97.478	4	-404.091	1	-14.827	5	-.012	1	-.081	5	-.558	1
151		19	max	-3.559	12	195.379	3	140.023	1	.007	3	.212	1	0	1
152			min	-109.169	4	-576.355	1	-13.198	5	-.012	1	-.097	5	0	5
153	M2	1	max	1105.913	1	2.332	4	1.261	1	0	3	0	3	0	1
154			min	-847.91	3	.571	15	-80.663	4	0	4	0	1	0	1
155		2	max	1106.241	1	2.316	4	1.261	1	0	3	0	1	0	15
156			min	-847.664	3	.568	15	-80.948	4	0	4	-.018	4	0	4
157		3	max	1106.57	1	2.301	4	1.261	1	0	3	0	1	0	15
158			min	-847.418	3	.564	15	-81.233	4	0	4	-.036	4	-.001	4
159		4	max	1106.898	1	2.286	4	1.261	1	0	3	0	1	0	15
160			min	-847.171	3	.561	15	-81.518	4	0	4	-.054	4	-.002	4
161		5	max	1107.226	1	2.271	4	1.261	1	0	3	.001	1	0	15
162			min	-846.925	3	.557	15	-81.803	4	0	4	-.072	4	-.002	4
163		6	max	1107.555	1	2.255	4	1.261	1	0	3	.001	1	0	15
164			min	-846.679	3	.553	15	-82.087	4	0	4	-.09	4	-.003	4
165		7	max	1107.883	1	2.24	4	1.261	1	0	3	.002	1	0	15
166			min	-846.433	3	.55	15	-82.372	4	0	4	-.108	4	-.003	4
167		8	max	1108.212	1	2.225	4	1.261	1	0	3	.002	1	0	15
168			min	-846.186	3	.546	15	-82.657	4	0	4	-.127	4	-.004	4
169		9	max	1108.54	1	2.21	4	1.261	1	0	3	.002	1	0	15
170			min	-845.94	3	.543	15	-82.942	4	0	4	-.145	4	-.004	4
171		10	max	1108.869	1	2.194	4	1.261	1	0	3	.002	1	-.001	15
172			min	-845.694	3	.539	15	-83.227	4	0	4	-.163	4	-.005	4
173		11	max	1109.197	1	2.179	4	1.261	1	0	3	.003	1	-.001	15
174			min	-845.447	3	.536	15	-83.511	4	0	4	-.182	4	-.005	4
175		12	max	1109.526	1	2.164	4	1.261	1	0	3	.003	1	-.001	15
176			min	-845.201	3	.532	15	-83.796	4	0	4	-.2	4	-.005	4
177		13	max	1109.854	1	2.149	4	1.261	1	0	3	.003	1	-.001	15
178			min	-844.955	3	.528	15	-84.081	4	0	4	-.219	4	-.006	4
179		14	max	1110.182	1	2.133	4	1.261	1	0	3	.004	1	-.002	15
180			min	-844.708	3	.525	15	-84.366	4	0	4	-.238	4	-.006	4
181		15	max	1110.511	1	2.118	4	1.261	1	0	3	.004	1	-.002	15
182			min	-844.462	3	.521	15	-84.651	4	0	4	-.256	4	-.007	4
183		16	max	1110.839	1	2.103	4	1.261	1	0	3	.004	1	-.002	15
184			min	-844.216	3	.518	15	-84.936	4	0	4	-.275	4	-.007	4
185		17	max	1111.168	1	2.087	4	1.261	1	0	3	.004	1	-.002	15
186			min	-843.969	3	.514	15	-85.22	4	0	4	-.294	4	-.008	4
187		18	max	1111.496	1	2.072	4	1.261	1	0	3	.005	1	-.002	15
188			min	-843.723	3	.51	15	-85.505	4	0	4	-.313	4	-.008	4
189		19	max	1111.825	1	2.057	4	1.261	1	0	3	.005	1	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-843.477	3	.507	15	-85.79	4	0	4	-.332	4	-.009	4
191	M3	1	max	230.979	2	8.105	4	.012	1	0	3	0	1	.009	4
192			min	-337.413	3	1.917	15	-1.215	5	0	4	-.012	4	.002	15
193		2	max	230.808	2	7.332	4	.012	1	0	3	0	1	.005	4
194			min	-337.541	3	1.736	15	-.673	5	0	4	-.013	4	.001	15
195		3	max	230.638	2	6.56	4	.012	1	0	3	0	1	.003	2
196			min	-337.669	3	1.554	15	-.131	5	0	4	-.013	4	0	12
197		4	max	230.468	2	5.787	4	.465	4	0	3	0	1	0	2
198			min	-337.796	3	1.373	15	0	12	0	4	-.013	4	-.001	3
199		5	max	230.297	2	5.015	4	1.007	4	0	3	0	1	0	15
200			min	-337.924	3	1.191	15	0	12	0	4	-.013	4	-.002	6
201		6	max	230.127	2	4.243	4	1.549	4	0	3	0	1	0	15
202			min	-338.052	3	1.01	15	0	12	0	4	-.012	4	-.004	6
203		7	max	229.957	2	3.47	4	2.091	4	0	3	0	1	-.001	15
204			min	-338.18	3	.828	15	0	12	0	4	-.011	4	-.006	6
205		8	max	229.786	2	2.698	4	2.633	4	0	3	0	1	-.002	15
206			min	-338.307	3	.646	15	0	12	0	4	-.01	4	-.007	6
207		9	max	229.616	2	1.925	4	3.175	4	0	3	0	1	-.002	15
208			min	-338.435	3	.465	15	0	12	0	4	-.009	4	-.008	6
209		10	max	229.446	2	1.153	4	3.718	4	0	3	0	1	-.002	15
210			min	-338.563	3	.283	15	0	12	0	4	-.008	5	-.009	6
211		11	max	229.275	2	.381	2	4.26	4	0	3	0	1	-.002	15
212			min	-338.691	3	.046	12	0	12	0	4	-.006	5	-.009	6
213		12	max	229.105	2	-.08	15	4.802	4	0	3	0	1	-.002	15
214			min	-338.819	3	-.393	6	0	12	0	4	-.004	5	-.009	6
215		13	max	228.934	2	-.261	15	5.344	4	0	3	0	1	-.002	15
216			min	-338.946	3	-1.165	6	0	12	0	4	-.002	5	-.009	6
217		14	max	228.764	2	-.443	15	5.886	4	0	3	0	4	-.002	15
218			min	-339.074	3	-1.938	6	0	12	0	4	0	12	-.008	6
219		15	max	228.594	2	-.625	15	6.428	4	0	3	.003	4	-.002	15
220			min	-339.202	3	-2.71	6	0	12	0	4	0	12	-.007	6
221		16	max	228.423	2	-.806	15	6.97	4	0	3	.006	4	-.001	15
222			min	-339.33	3	-3.483	6	0	12	0	4	0	12	-.006	6
223		17	max	228.253	2	-.988	15	7.512	4	0	3	.009	4	0	15
224			min	-339.457	3	-4.255	6	0	12	0	4	0	12	-.004	6
225		18	max	228.083	2	-1.169	15	8.055	4	0	3	.012	4	0	15
226			min	-339.585	3	-5.028	6	0	12	0	4	0	12	-.002	6
227		19	max	227.912	2	-1.351	15	8.597	4	0	3	.016	4	0	1
228			min	-339.713	3	-5.8	6	0	12	0	4	0	12	0	1
229	M4	1	max	1229.507	1	0	1	-.318	12	0	1	.008	4	0	1
230			min	-250.52	3	0	1	-240.299	4	0	1	0	10	0	1
231		2	max	1229.677	1	0	1	-.318	12	0	1	0	12	0	1
232			min	-250.392	3	0	1	-240.447	4	0	1	-.019	4	0	1
233		3	max	1229.848	1	0	1	-.318	12	0	1	0	12	0	1
234			min	-250.265	3	0	1	-240.594	4	0	1	-.047	4	0	1
235		4	max	1230.018	1	0	1	-.318	12	0	1	0	12	0	1
236			min	-250.137	3	0	1	-240.742	4	0	1	-.075	4	0	1
237		5	max	1230.188	1	0	1	-.318	12	0	1	0	12	0	1
238			min	-250.009	3	0	1	-240.89	4	0	1	-.102	4	0	1
239		6	max	1230.359	1	0	1	-.318	12	0	1	0	12	0	1
240			min	-249.881	3	0	1	-241.037	4	0	1	-.13	4	0	1
241		7	max	1230.529	1	0	1	-.318	12	0	1	0	12	0	1
242			min	-249.754	3	0	1	-241.185	4	0	1	-.158	4	0	1
243		8	max	1230.699	1	0	1	-.318	12	0	1	0	12	0	1
244			min	-249.626	3	0	1	-241.333	4	0	1	-.185	4	0	1
245		9	max	1230.87	1	0	1	-.318	12	0	1	0	12	0	1
246			min	-249.498	3	0	1	-241.48	4	0	1	-.213	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1231.04	1	0	1	-.318	12	0	1	0	12	0	1
248		min	-249.37	3	0	1	-241.628	4	0	1	-.241	4	0	1
249	11	max	1231.21	1	0	1	-.318	12	0	1	0	12	0	1
250		min	-249.242	3	0	1	-241.775	4	0	1	-.268	4	0	1
251	12	max	1231.381	1	0	1	-.318	12	0	1	0	12	0	1
252		min	-249.115	3	0	1	-241.923	4	0	1	-.296	4	0	1
253	13	max	1231.551	1	0	1	-.318	12	0	1	0	12	0	1
254		min	-248.987	3	0	1	-242.071	4	0	1	-.324	4	0	1
255	14	max	1231.721	1	0	1	-.318	12	0	1	0	12	0	1
256		min	-248.859	3	0	1	-242.218	4	0	1	-.352	4	0	1
257	15	max	1231.892	1	0	1	-.318	12	0	1	0	12	0	1
258		min	-248.731	3	0	1	-242.366	4	0	1	-.38	4	0	1
259	16	max	1232.062	1	0	1	-.318	12	0	1	0	12	0	1
260		min	-248.604	3	0	1	-242.514	4	0	1	-.407	4	0	1
261	17	max	1232.232	1	0	1	-.318	12	0	1	0	12	0	1
262		min	-248.476	3	0	1	-242.661	4	0	1	-.435	4	0	1
263	18	max	1232.403	1	0	1	-.318	12	0	1	0	12	0	1
264		min	-248.348	3	0	1	-242.809	4	0	1	-.463	4	0	1
265	19	max	1232.573	1	0	1	-.318	12	0	1	0	12	0	1
266		min	-248.22	3	0	1	-242.956	4	0	1	-.491	4	0	1
267	M6	1	max	3566.039	1	2.569	2	0	1	0	0	4	0	1
268		min	-2786.996	3	.326	12	-81.388	4	0	4	0	1	0	1
269	2	max	3566.368	1	2.557	2	0	1	0	1	0	1	0	12
270		min	-2786.75	3	.32	12	-81.673	4	0	4	-.018	4	0	2
271	3	max	3566.696	1	2.545	2	0	1	0	1	0	1	0	12
272		min	-2786.504	3	.314	12	-81.957	4	0	4	-.036	4	-.001	2
273	4	max	3567.024	1	2.534	2	0	1	0	1	0	1	0	12
274		min	-2786.258	3	.308	12	-82.242	4	0	4	-.054	4	-.002	2
275	5	max	3567.353	1	2.522	2	0	1	0	1	0	1	0	12
276		min	-2786.011	3	.302	12	-82.527	4	0	4	-.073	4	-.002	2
277	6	max	3567.681	1	2.51	2	0	1	0	1	0	1	0	12
278		min	-2785.765	3	.297	12	-82.812	4	0	4	-.091	4	-.003	2
279	7	max	3568.01	1	2.498	2	0	1	0	1	0	1	0	12
280		min	-2785.519	3	.291	12	-83.097	4	0	4	-.109	4	-.003	2
281	8	max	3568.338	1	2.486	2	0	1	0	1	0	1	0	12
282		min	-2785.272	3	.285	12	-83.382	4	0	4	-.128	4	-.004	2
283	9	max	3568.667	1	2.474	2	0	1	0	1	0	1	0	12
284		min	-2785.026	3	.279	12	-83.666	4	0	4	-.146	4	-.004	2
285	10	max	3568.995	1	2.462	2	0	1	0	1	0	1	0	12
286		min	-2784.78	3	.273	12	-83.951	4	0	4	-.165	4	-.005	2
287	11	max	3569.323	1	2.45	2	0	1	0	1	0	1	0	12
288		min	-2784.533	3	.267	12	-84.236	4	0	4	-.183	4	-.006	2
289	12	max	3569.652	1	2.438	2	0	1	0	1	0	1	0	12
290		min	-2784.287	3	.261	12	-84.521	4	0	4	-.202	4	-.006	2
291	13	max	3569.98	1	2.427	2	0	1	0	1	0	1	0	12
292		min	-2784.041	3	.255	12	-84.806	4	0	4	-.221	4	-.007	2
293	14	max	3570.309	1	2.415	2	0	1	0	1	0	1	0	12
294		min	-2783.794	3	.249	12	-85.091	4	0	4	-.24	4	-.007	2
295	15	max	3570.637	1	2.403	2	0	1	0	1	0	1	0	12
296		min	-2783.548	3	.243	12	-85.375	4	0	4	-.259	4	-.008	2
297	16	max	3570.966	1	2.391	2	0	1	0	1	0	1	0	12
298		min	-2783.302	3	.237	12	-85.66	4	0	4	-.278	4	-.008	2
299	17	max	3571.294	1	2.379	2	0	1	0	1	0	1	0	12
300		min	-2783.055	3	.231	12	-85.945	4	0	4	-.297	4	-.009	2
301	18	max	3571.623	1	2.367	2	0	1	0	1	0	1	-.001	12
302		min	-2782.809	3	.225	12	-86.23	4	0	4	-.316	4	-.009	2
303	19	max	3571.951	1	2.355	2	0	1	0	1	0	1	-.001	12



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-2782.563	3	.219	12	-86.515	4	0	4	-.335	4	-.01	2
305	M7	1	max	1002.926	2	8.119	6	0	1	0	1	0	1	.01	2
306			min	-1056.506	3	1.905	15	-1.285	5	0	4	-.013	4	.001	12
307		2	max	1002.756	2	7.346	6	0	1	0	1	0	1	.007	2
308			min	-1056.634	3	1.723	15	-.743	5	0	4	-.013	4	0	3
309		3	max	1002.585	2	6.574	6	0	1	0	1	0	1	.005	2
310			min	-1056.761	3	1.542	15	-.201	5	0	4	-.013	4	-.002	3
311		4	max	1002.415	2	5.801	6	.387	4	0	1	0	1	.002	2
312			min	-1056.889	3	1.36	15	0	1	0	4	-.013	4	-.003	3
313		5	max	1002.244	2	5.029	6	.929	4	0	1	0	1	0	2
314			min	-1057.017	3	1.178	15	0	1	0	4	-.013	4	-.004	3
315		6	max	1002.074	2	4.257	6	1.471	4	0	1	0	1	-.001	15
316			min	-1057.145	3	.997	15	0	1	0	4	-.012	4	-.005	3
317		7	max	1001.904	2	3.484	6	2.013	4	0	1	0	1	-.001	15
318			min	-1057.273	3	.815	15	0	1	0	4	-.012	4	-.006	3
319		8	max	1001.733	2	2.712	6	2.556	4	0	1	0	1	-.002	15
320			min	-1057.4	3	.634	15	0	1	0	4	-.011	4	-.007	4
321		9	max	1001.563	2	1.939	6	3.098	4	0	1	0	1	-.002	15
322			min	-1057.528	3	.444	12	0	1	0	4	-.009	4	-.008	4
323		10	max	1001.393	2	1.294	2	3.64	4	0	1	0	1	-.002	15
324			min	-1057.656	3	.143	12	0	1	0	4	-.008	4	-.009	4
325		11	max	1001.222	2	.692	2	4.182	4	0	1	0	1	-.002	15
326			min	-1057.784	3	-.267	3	0	1	0	4	-.006	4	-.009	4
327		12	max	1001.052	2	.09	2	4.724	4	0	1	0	1	-.002	15
328			min	-1057.911	3	-.718	3	0	1	0	4	-.005	4	-.009	4
329		13	max	1000.882	2	-.274	15	5.266	4	0	1	0	1	-.002	15
330			min	-1058.039	3	-1.169	3	0	1	0	4	-.002	4	-.009	4
331		14	max	1000.711	2	-.456	15	5.808	4	0	1	0	1	-.002	15
332			min	-1058.167	3	-1.923	4	0	1	0	4	0	5	-.008	4
333		15	max	1000.541	2	-.637	15	6.35	4	0	1	.002	4	-.002	15
334			min	-1058.295	3	-2.695	4	0	1	0	4	0	1	-.007	4
335		16	max	1000.371	2	-.819	15	6.893	4	0	1	.005	4	-.001	15
336			min	-1058.422	3	-3.468	4	0	1	0	4	0	1	-.006	4
337		17	max	1000.2	2	-1	15	7.435	4	0	1	.008	4	0	15
338			min	-1058.55	3	-4.24	4	0	1	0	4	0	1	-.004	4
339		18	max	1000.03	2	-1.182	15	7.977	4	0	1	.012	4	0	15
340			min	-1058.678	3	-5.012	4	0	1	0	4	0	1	-.002	4
341		19	max	999.86	2	-1.363	15	8.519	4	0	1	.015	4	0	1
342			min	-1058.806	3	-5.785	4	0	1	0	4	0	1	0	1
343	M8	1	max	3605.558	1	0	1	0	1	0	1	.008	4	0	1
344			min	-869.469	3	0	1	-234.915	4	0	1	0	1	0	1
345		2	max	3605.729	1	0	1	0	1	0	1	0	1	0	1
346			min	-869.341	3	0	1	-235.062	4	0	1	-.019	4	0	1
347		3	max	3605.899	1	0	1	0	1	0	1	0	1	0	1
348			min	-869.214	3	0	1	-235.21	4	0	1	-.046	4	0	1
349		4	max	3606.069	1	0	1	0	1	0	1	0	1	0	1
350			min	-869.086	3	0	1	-235.358	4	0	1	-.073	4	0	1
351		5	max	3606.24	1	0	1	0	1	0	1	0	1	0	1
352			min	-868.958	3	0	1	-235.505	4	0	1	-.1	4	0	1
353		6	max	3606.41	1	0	1	0	1	0	1	0	1	0	1
354			min	-868.83	3	0	1	-235.653	4	0	1	-.127	4	0	1
355		7	max	3606.58	1	0	1	0	1	0	1	0	1	0	1
356			min	-868.703	3	0	1	-235.801	4	0	1	-.154	4	0	1
357		8	max	3606.751	1	0	1	0	1	0	1	0	1	0	1
358			min	-868.575	3	0	1	-235.948	4	0	1	-.181	4	0	1
359		9	max	3606.921	1	0	1	0	1	0	1	0	1	0	1
360			min	-868.447	3	0	1	-236.096	4	0	1	-.208	4	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361	10	max	3607.091	1	0	1	0	1	0	1	0	1	0	1
362		min	-868.319	3	0	1	-236.243	4	0	1	-.235	4	0	1
363	11	max	3607.262	1	0	1	0	1	0	1	0	1	0	1
364		min	-868.191	3	0	1	-236.391	4	0	1	-.263	4	0	1
365	12	max	3607.432	1	0	1	0	1	0	1	0	1	0	1
366		min	-868.064	3	0	1	-236.539	4	0	1	-.29	4	0	1
367	13	max	3607.602	1	0	1	0	1	0	1	0	1	0	1
368		min	-867.936	3	0	1	-236.686	4	0	1	-.317	4	0	1
369	14	max	3607.773	1	0	1	0	1	0	1	0	1	0	1
370		min	-867.808	3	0	1	-236.834	4	0	1	-.344	4	0	1
371	15	max	3607.943	1	0	1	0	1	0	1	0	1	0	1
372		min	-867.68	3	0	1	-236.982	4	0	1	-.371	4	0	1
373	16	max	3608.113	1	0	1	0	1	0	1	0	1	0	1
374		min	-867.553	3	0	1	-237.129	4	0	1	-.399	4	0	1
375	17	max	3608.284	1	0	1	0	1	0	1	0	1	0	1
376		min	-867.425	3	0	1	-237.277	4	0	1	-.426	4	0	1
377	18	max	3608.454	1	0	1	0	1	0	1	0	1	0	1
378		min	-867.297	3	0	1	-237.425	4	0	1	-.453	4	0	1
379	19	max	3608.624	1	0	1	0	1	0	1	0	1	0	1
380		min	-867.169	3	0	1	-237.572	4	0	1	-.48	4	0	1
381	M10	1	max	1105.913	1	2.229	6	-.046	12	0	1	0	1	0
382		min	-847.91	3	.503	15	-81.296	4	0	5	0	3	0	1
383	2	max	1106.241	1	2.214	6	-.046	12	0	1	0	10	0	15
384		min	-847.664	3	.499	15	-81.581	4	0	5	-.018	4	0	6
385	3	max	1106.57	1	2.199	6	-.046	12	0	1	0	12	0	15
386		min	-847.418	3	.496	15	-81.865	4	0	5	-.036	4	0	6
387	4	max	1106.898	1	2.184	6	-.046	12	0	1	0	12	0	15
388		min	-847.171	3	.492	15	-82.15	4	0	5	-.054	4	-.001	6
389	5	max	1107.226	1	2.168	6	-.046	12	0	1	0	12	0	15
390		min	-846.925	3	.489	15	-82.435	4	0	5	-.073	4	-.002	6
391	6	max	1107.555	1	2.153	6	-.046	12	0	1	0	12	0	15
392		min	-846.679	3	.485	15	-82.72	4	0	5	-.091	4	-.002	6
393	7	max	1107.883	1	2.138	6	-.046	12	0	1	0	12	0	15
394		min	-846.433	3	.481	15	-83.005	4	0	5	-.109	4	-.003	6
395	8	max	1108.212	1	2.123	6	-.046	12	0	1	0	12	0	15
396		min	-846.186	3	.478	15	-83.29	4	0	5	-.128	4	-.003	6
397	9	max	1108.54	1	2.107	6	-.046	12	0	1	0	12	0	15
398		min	-845.94	3	.474	15	-83.574	4	0	5	-.146	4	-.004	6
399	10	max	1108.869	1	2.092	6	-.046	12	0	1	0	12	0	15
400		min	-845.694	3	.471	15	-83.859	4	0	5	-.165	4	-.004	6
401	11	max	1109.197	1	2.077	6	-.046	12	0	1	0	12	-.001	15
402		min	-845.447	3	.467	15	-84.144	4	0	5	-.183	4	-.005	6
403	12	max	1109.526	1	2.062	6	-.046	12	0	1	0	12	-.001	15
404		min	-845.201	3	.463	15	-84.429	4	0	5	-.202	4	-.005	6
405	13	max	1109.854	1	2.046	6	-.046	12	0	1	0	12	-.001	15
406		min	-844.955	3	.46	15	-84.714	4	0	5	-.221	4	-.006	6
407	14	max	1110.182	1	2.031	6	-.046	12	0	1	0	12	-.001	15
408		min	-844.708	3	.456	15	-84.999	4	0	5	-.239	4	-.006	6
409	15	max	1110.511	1	2.016	6	-.046	12	0	1	0	12	-.001	15
410		min	-844.462	3	.453	15	-85.283	4	0	5	-.258	4	-.007	6
411	16	max	1110.839	1	2.001	6	-.046	12	0	1	0	12	-.002	15
412		min	-844.216	3	.449	15	-85.568	4	0	5	-.277	4	-.007	6
413	17	max	1111.168	1	1.985	6	-.046	12	0	1	0	12	-.002	15
414		min	-843.969	3	.445	15	-85.853	4	0	5	-.296	4	-.007	6
415	18	max	1111.496	1	1.97	6	-.046	12	0	1	0	12	-.002	15
416		min	-843.723	3	.442	15	-86.138	4	0	5	-.315	4	-.008	6
417	19	max	1111.825	1	1.955	6	-.046	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-843.477	3	.438	15	-86.423	4	0	5	-.334	4	-.008	6
419	M11	1	max	230.979	2	8.051	6	0	12	0	1	0	12	.008	6
420			min	-337.413	3	1.881	15	-1.216	5	0	4	-.013	4	.002	15
421		2	max	230.808	2	7.279	6	0	12	0	1	0	12	.005	6
422			min	-337.541	3	1.7	15	-.674	5	0	4	-.013	4	.001	15
423		3	max	230.638	2	6.506	6	0	12	0	1	0	12	.003	2
424			min	-337.669	3	1.518	15	-.132	5	0	4	-.013	4	0	12
425		4	max	230.468	2	5.734	6	.459	4	0	1	0	12	0	2
426			min	-337.796	3	1.337	15	-.012	1	0	4	-.013	4	-.001	3
427		5	max	230.297	2	4.961	6	1.001	4	0	1	0	12	0	15
428			min	-337.924	3	1.155	15	-.012	1	0	4	-.013	4	-.003	4
429		6	max	230.127	2	4.189	6	1.543	4	0	1	0	12	-.001	15
430			min	-338.052	3	.974	15	-.012	1	0	4	-.012	4	-.005	4
431		7	max	229.957	2	3.416	6	2.085	4	0	1	0	12	-.002	15
432			min	-338.18	3	.792	15	-.012	1	0	4	-.011	4	-.006	4
433		8	max	229.786	2	2.644	6	2.628	4	0	1	0	12	-.002	15
434			min	-338.307	3	.61	15	-.012	1	0	4	-.01	4	-.007	4
435		9	max	229.616	2	1.872	6	3.17	4	0	1	0	12	-.002	15
436			min	-338.435	3	.429	15	-.012	1	0	4	-.009	4	-.008	4
437		10	max	229.446	2	1.099	6	3.712	4	0	1	0	12	-.002	15
438			min	-338.563	3	.247	15	-.012	1	0	4	-.008	4	-.009	4
439		11	max	229.275	2	.381	2	4.254	4	0	1	0	12	-.002	15
440			min	-338.691	3	.046	12	-.012	1	0	4	-.006	4	-.009	4
441		12	max	229.105	2	-.116	15	4.796	4	0	1	0	12	-.002	15
442			min	-338.819	3	-.447	4	-.012	1	0	4	-.004	4	-.009	4
443		13	max	228.934	2	-.297	15	5.338	4	0	1	0	12	-.002	15
444			min	-338.946	3	-1.219	4	-.012	1	0	4	-.002	4	-.009	4
445		14	max	228.764	2	-.479	15	5.88	4	0	1	0	5	-.002	15
446			min	-339.074	3	-1.992	4	-.012	1	0	4	0	1	-.008	4
447		15	max	228.594	2	-.661	15	6.422	4	0	1	.003	4	-.002	15
448			min	-339.202	3	-2.764	4	-.012	1	0	4	0	1	-.007	4
449		16	max	228.423	2	-.842	15	6.965	4	0	1	.006	4	-.001	15
450			min	-339.33	3	-3.536	4	-.012	1	0	4	0	1	-.006	4
451		17	max	228.253	2	-1.024	15	7.507	4	0	1	.009	4	-.001	15
452			min	-339.457	3	-4.309	4	-.012	1	0	4	0	1	-.004	4
453		18	max	228.083	2	-1.205	15	8.049	4	0	1	.012	4	0	15
454			min	-339.585	3	-5.081	4	-.012	1	0	4	0	1	-.002	4
455		19	max	227.912	2	-1.387	15	8.591	4	0	1	.016	4	0	1
456			min	-339.713	3	-5.854	4	-.012	1	0	4	0	1	0	1
457	M12	1	max	1229.507	1	0	1	8.414	1	0	1	.008	4	0	1
458			min	-250.52	3	0	1	-236.067	4	0	1	0	1	0	1
459		2	max	1229.677	1	0	1	8.414	1	0	1	0	1	0	1
460			min	-250.392	3	0	1	-236.214	4	0	1	-.019	4	0	1
461		3	max	1229.848	1	0	1	8.414	1	0	1	.002	1	0	1
462			min	-250.265	3	0	1	-236.362	4	0	1	-.046	4	0	1
463		4	max	1230.018	1	0	1	8.414	1	0	1	.003	1	0	1
464			min	-250.137	3	0	1	-236.509	4	0	1	-.073	4	0	1
465		5	max	1230.188	1	0	1	8.414	1	0	1	.004	1	0	1
466			min	-250.009	3	0	1	-236.657	4	0	1	-.1	4	0	1
467		6	max	1230.359	1	0	1	8.414	1	0	1	.005	1	0	1
468			min	-249.881	3	0	1	-236.805	4	0	1	-.127	4	0	1
469		7	max	1230.529	1	0	1	8.414	1	0	1	.006	1	0	1
470			min	-249.754	3	0	1	-236.952	4	0	1	-.155	4	0	1
471		8	max	1230.699	1	0	1	8.414	1	0	1	.007	1	0	1
472			min	-249.626	3	0	1	-237.1	4	0	1	-.182	4	0	1
473		9	max	1230.87	1	0	1	8.414	1	0	1	.008	1	0	1
474			min	-249.498	3	0	1	-237.248	4	0	1	-.209	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	1231.04	1	0	1	8.414	1	0	1	.009	1	0	1
476		min	-249.37	3	0	1	-237.395	4	0	1	-.236	4	0	1
477	11	max	1231.21	1	0	1	8.414	1	0	1	.01	1	0	1
478		min	-249.242	3	0	1	-237.543	4	0	1	-.264	4	0	1
479	12	max	1231.381	1	0	1	8.414	1	0	1	.01	1	0	1
480		min	-249.115	3	0	1	-237.691	4	0	1	-.291	4	0	1
481	13	max	1231.551	1	0	1	8.414	1	0	1	.011	1	0	1
482		min	-248.987	3	0	1	-237.838	4	0	1	-.318	4	0	1
483	14	max	1231.721	1	0	1	8.414	1	0	1	.012	1	0	1
484		min	-248.859	3	0	1	-237.986	4	0	1	-.346	4	0	1
485	15	max	1231.892	1	0	1	8.414	1	0	1	.013	1	0	1
486		min	-248.731	3	0	1	-238.133	4	0	1	-.373	4	0	1
487	16	max	1232.062	1	0	1	8.414	1	0	1	.014	1	0	1
488		min	-248.604	3	0	1	-238.281	4	0	1	-.4	4	0	1
489	17	max	1232.232	1	0	1	8.414	1	0	1	.015	1	0	1
490		min	-248.476	3	0	1	-238.429	4	0	1	-.428	4	0	1
491	18	max	1232.403	1	0	1	8.414	1	0	1	.016	1	0	1
492		min	-248.348	3	0	1	-238.576	4	0	1	-.455	4	0	1
493	19	max	1232.573	1	0	1	8.414	1	0	1	.017	1	0	1
494		min	-248.22	3	0	1	-238.724	4	0	1	-.482	4	0	1
495	M1	1	max	139.855	1	481.636	3	57.184	5	0	.211	1	0	3
496		min	-5.96	5	-509.49	1	-88.69	1	0	3	-.058	5	-.013	1
497	2	max	140.225	1	480.599	3	58.426	5	0	1	.164	1	.256	1
498		min	-5.787	5	-510.874	1	-88.69	1	0	3	-.028	5	-.253	3
499	3	max	198.505	3	563.671	1	-3.461	12	0	3	.117	1	.513	1
500		min	-129.094	2	-347.455	3	-87.65	1	0	1	.001	15	-.497	3
501	4	max	198.783	3	562.287	1	-3.461	12	0	3	.071	1	.216	1
502		min	-128.723	2	-348.493	3	-87.65	1	0	1	-.008	5	-.313	3
503	5	max	199.061	3	560.904	1	-3.461	12	0	3	.025	1	-.004	15
504		min	-128.353	2	-349.53	3	-87.65	1	0	1	-.017	5	-.129	3
505	6	max	199.339	3	559.52	1	-3.461	12	0	3	0	12	.056	3
506		min	-127.982	2	-350.568	3	-87.65	1	0	1	-.029	4	-.376	1
507	7	max	199.617	3	558.136	1	-3.461	12	0	3	-.003	12	.241	3
508		min	-127.611	2	-351.606	3	-87.65	1	0	1	-.068	1	-.671	1
509	8	max	199.895	3	556.753	1	-3.461	12	0	3	-.004	12	.427	3
510		min	-127.24	2	-352.643	3	-87.65	1	0	1	-.114	1	-.965	1
511	9	max	207.695	3	33.4	2	39.23	5	0	9	.067	1	.499	3
512		min	-71.404	2	.418	15	-128.447	1	0	3	-.119	5	-.1.1	1
513	10	max	207.974	3	32.016	2	40.472	5	0	9	0	12	.486	3
514		min	-71.033	2	0	5	-128.447	1	0	3	-.099	4	-1.109	1
515	11	max	208.252	3	30.632	2	41.713	5	0	9	-.003	12	.473	3
516		min	-70.662	2	-1.732	4	-128.447	1	0	3	-.09	4	-1.117	1
517	12	max	216.015	3	231.829	3	127.621	5	0	1	.112	1	.412	3
518		min	-47.296	5	-591.87	1	-85.631	1	0	3	-.171	5	-.986	1
519	13	max	216.293	3	230.791	3	128.862	5	0	1	.067	1	.29	3
520		min	-47.123	5	-593.253	1	-85.631	1	0	3	-.103	5	-.673	1
521	14	max	216.571	3	229.753	3	130.103	5	0	1	.022	1	.168	3
522		min	-46.95	5	-594.637	1	-85.631	1	0	3	-.035	5	-.36	1
523	15	max	216.849	3	228.716	3	131.345	5	0	1	.034	5	.047	3
524		min	-46.777	5	-596.02	1	-85.631	1	0	3	-.023	1	-.046	1
525	16	max	217.127	3	227.678	3	132.586	5	0	1	.104	5	.269	1
526		min	-46.604	5	-597.404	1	-85.631	1	0	3	-.068	1	-.073	3
527	17	max	217.405	3	226.64	3	133.828	5	0	1	.174	5	.585	1
528		min	-46.431	5	-598.788	1	-85.631	1	0	3	-.114	1	-.193	3
529	18	max	13.025	5	578.863	1	-3.559	12	0	5	.144	5	.293	1
530		min	-140.392	1	-194.368	3	-110.459	4	0	1	-.163	1	-.096	3
531	19	max	13.198	5	577.48	1	-3.559	12	0	5	.097	5	.007	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-140.022	1	-195.406	3	-109.217	4	0	1	-.212	1	-.012	1
533	M5	max	301.772	1	1606.112	3	87.128	5	0	1	0	1	.027	1
534		min	9.232	12	-1723.345	1	0	1	0	4	-.132	4	0	3
535		max	302.143	1	1605.075	3	88.369	5	0	1	0	1	.937	1
536		min	9.417	12	-1724.728	1	0	1	0	4	-.086	4	-.848	3
537		max	637.407	3	1729.138	1	13.693	4	0	4	0	1	1.805	1
538		min	-493.3	1	-1120.083	3	0	1	0	1	-.04	4	-1.662	3
539		max	637.685	3	1727.755	1	14.935	4	0	4	0	1	.893	1
540		min	-492.929	1	-1121.121	3	0	1	0	1	-.033	4	-1.071	3
541		max	637.963	3	1726.371	1	16.176	4	0	4	0	1	.013	9
542		min	-492.559	1	-1122.158	3	0	1	0	1	-.025	4	-.479	3
543		max	638.241	3	1724.988	1	17.418	4	0	4	0	1	.113	3
544		min	-492.188	1	-1123.196	3	0	1	0	1	-.016	5	-.929	1
545		max	638.519	3	1723.604	1	18.659	4	0	4	0	1	.706	3
546		min	-491.817	1	-1124.234	3	0	1	0	1	-.008	5	-1.838	1
547		max	638.798	3	1722.22	1	19.901	4	0	4	.004	4	1.3	3
548		min	-491.446	1	-1125.271	3	0	1	0	1	0	1	-2.747	1
549		max	652.464	3	110.481	2	125.337	4	0	1	0	1	1.497	3
550		min	-360.033	2	.418	15	0	1	0	1	-.156	4	-3.106	1
551		max	652.742	3	109.097	2	126.579	4	0	1	0	1	1.45	3
552		min	-359.662	2	0	15	0	1	0	1	-.09	5	-3.138	1
553		max	653.02	3	107.714	2	127.82	4	0	1	0	1	1.404	3
554		min	-359.291	2	-1.587	6	0	1	0	1	-.024	5	-3.168	1
555		max	666.759	3	734.753	3	173.729	4	0	1	0	1	1.232	3
556		min	-244.948	2	-1840.175	1	0	1	0	4	-.237	4	-2.821	1
557		max	667.037	3	733.716	3	174.971	4	0	1	0	1	.845	3
558		min	-244.577	2	-1841.559	1	0	1	0	4	-.145	4	-1.85	1
559		max	667.315	3	732.678	3	176.212	4	0	1	0	1	.458	3
560		min	-244.206	2	-1842.942	1	0	1	0	4	-.052	4	-.878	1
561		max	667.593	3	731.64	3	177.454	4	0	1	.041	4	.141	2
562		min	-243.835	2	-1844.326	1	0	1	0	4	0	1	-.004	13
563		max	667.871	3	730.603	3	178.695	4	0	1	.135	4	1.069	1
564		min	-243.465	2	-1845.71	1	0	1	0	4	0	1	-.314	3
565		max	668.149	3	729.565	3	179.937	4	0	1	.229	4	2.043	1
566		min	-243.094	2	-1847.093	1	0	1	0	4	0	1	-.7	3
567		max	-9.568	12	1956.054	1	0	1	0	4	.225	4	1.056	1
568		min	-301.809	1	-665.541	3	-42.097	5	0	1	0	1	-.366	3
569		max	-9.382	12	1954.67	1	0	1	0	4	.204	4	.024	1
570		min	-301.439	1	-666.579	3	-40.855	5	0	1	0	1	-.014	3
571	M9	max	139.855	1	481.636	3	88.69	1	0	3	-.008	12	0	3
572		min	4.861	12	-509.49	1	3.51	12	0	1	-.211	1	-.013	1
573		max	140.225	1	480.599	3	88.69	1	0	3	-.007	12	.256	1
574		min	5.047	12	-510.874	1	3.51	12	0	1	-.164	1	-.253	3
575		max	198.505	3	563.671	1	87.65	1	0	1	-.005	12	.513	1
576		min	-129.094	2	-347.455	3	-10.388	5	0	3	-.117	1	-.497	3
577		max	198.783	3	562.287	1	87.65	1	0	1	-.003	12	.216	1
578		min	-128.723	2	-348.493	3	-9.146	5	0	3	-.071	1	-.313	3
579		max	199.061	3	560.904	1	87.65	1	0	1	-.001	12	-.004	15
580		min	-128.353	2	-349.53	3	-7.905	5	0	3	-.025	1	-.129	3
581		max	199.339	3	559.52	1	87.65	1	0	1	.021	1	.056	3
582		min	-127.982	2	-350.568	3	-6.663	5	0	3	-.023	5	-.376	1
583		max	199.617	3	558.136	1	87.65	1	0	1	.068	1	.241	3
584		min	-127.611	2	-351.606	3	-5.422	5	0	3	-.026	5	-.671	1
585		max	199.895	3	556.753	1	87.65	1	0	1	.114	1	.427	3
586		min	-127.24	2	-352.643	3	-4.181	5	0	3	-.029	5	-.965	1
587		max	207.695	3	33.4	2	128.447	1	0	3	-.003	12	.499	3
588		min	-71.404	2	.422	15	4.971	12	0	9	-.141	4	-1.1	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	207.974	3	32.016	2	128.447	1	0	3	0	1	.486	3
590		min	-71.033	2	.005	15	4.971	12	0	9	-.099	4	-1.109	1
591	11	max	208.252	3	30.632	2	128.447	1	0	3	.069	1	.473	3
592		min	-70.662	2	-1.693	6	4.971	12	0	9	-.07	5	-1.117	1
593	12	max	216.015	3	231.829	3	155.213	4	0	3	-.004	12	.412	3
594		min	-44.799	10	-591.87	1	3.241	12	0	1	-.206	4	-.986	1
595	13	max	216.293	3	230.791	3	156.455	4	0	3	-.003	12	.29	3
596		min	-44.49	10	-593.253	1	3.241	12	0	1	-.124	4	-.673	1
597	14	max	216.571	3	229.753	3	157.696	4	0	3	0	12	.168	3
598		min	-44.181	10	-594.637	1	3.241	12	0	1	-.041	4	-.36	1
599	15	max	216.849	3	228.716	3	158.938	4	0	3	.042	4	.047	3
600		min	-43.872	10	-596.02	1	3.241	12	0	1	0	12	-.046	1
601	16	max	217.127	3	227.678	3	160.179	4	0	3	.126	4	.269	1
602		min	-43.563	10	-597.404	1	3.241	12	0	1	.003	12	-.073	3
603	17	max	217.405	3	226.64	3	161.421	4	0	3	.211	4	.585	1
604		min	-43.254	10	-598.788	1	3.241	12	0	1	.004	12	-.193	3
605	18	max	-4.972	12	578.863	1	93.756	1	0	1	.196	4	.293	1
606		min	-140.392	1	-194.368	3	-81.428	5	0	3	.006	12	-.096	3
607	19	max	-4.786	12	577.48	1	93.756	1	0	1	.212	1	.007	3
608		min	-140.022	1	-195.406	3	-80.186	5	0	3	.008	12	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.111	1	.004	3	8.819e-3	1	NC	1	NC	1
2			min	-484	4	-.014	3	-.001	10	-1.079e-3	3	NC	1	NC	1
3		2	max	0	1	.23	3	.035	1	1.016e-2	1	NC	5	NC	2
4			min	-484	4	-.126	1	-.014	5	-1.126e-3	3	1006.698	3	7360.682	1
5		3	max	0	1	.428	3	.084	1	1.151e-2	1	NC	5	NC	3
6			min	-484	4	-.314	1	-.016	5	-1.173e-3	3	556.526	3	3000.503	1
7		4	max	0	1	.547	3	.126	1	1.285e-2	1	NC	5	NC	3
8			min	-484	4	-.42	1	-.011	5	-1.221e-3	3	438.008	3	1981.172	1
9		5	max	0	1	.574	3	.148	1	1.42e-2	1	NC	5	NC	3
10			min	-484	4	-.428	1	-.002	5	-1.268e-3	3	417.975	3	1684.348	1
11		6	max	0	1	.511	3	.143	1	1.554e-2	1	NC	5	NC	3
12			min	-484	4	-.341	1	.005	15	-1.315e-3	3	468.668	3	1740.707	1
13		7	max	0	1	.375	3	.113	1	1.689e-2	1	NC	5	NC	3
14			min	-484	4	-.181	1	.007	10	-1.362e-3	3	631.594	3	2210.589	1
15		8	max	0	1	.204	3	.067	1	1.823e-2	1	NC	4	NC	2
16			min	-484	4	-.003	9	.002	10	-1.409e-3	3	1128.844	3	3793.866	1
17		9	max	0	1	.188	1	.02	1	1.958e-2	1	NC	4	NC	1
18			min	-484	4	.005	15	-.002	10	-1.456e-3	3	3189.08	1	NC	1
19		10	max	0	1	.266	1	.011	3	2.092e-2	1	NC	3	NC	1
20			min	-484	4	-.022	3	-.007	2	-1.504e-3	3	1587.09	1	NC	1
21		11	max	0	12	.188	1	.02	1	1.958e-2	1	NC	4	NC	1
22			min	-484	4	.005	15	-.011	5	-1.456e-3	3	3189.08	1	NC	1
23		12	max	0	12	.204	3	.067	1	1.823e-2	1	NC	4	NC	2
24			min	-484	4	-.003	9	-.011	5	-1.409e-3	3	1128.844	3	3793.866	1
25		13	max	0	12	.375	3	.113	1	1.689e-2	1	NC	5	NC	3
26			min	-484	4	-.181	1	-.004	5	-1.362e-3	3	631.594	3	2210.589	1
27		14	max	0	12	.511	3	.143	1	1.554e-2	1	NC	5	NC	3
28			min	-484	4	-.341	1	.004	15	-1.315e-3	3	468.668	3	1740.707	1
29		15	max	0	12	.574	3	.148	1	1.42e-2	1	NC	5	NC	3
30			min	-485	4	-.428	1	.009	12	-1.268e-3	3	417.975	3	1684.348	1
31		16	max	0	12	.547	3	.126	1	1.285e-2	1	NC	5	NC	3
32			min	-485	4	-.42	1	.008	12	-1.221e-3	3	438.008	3	1981.172	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.428	3	.084	1	1.151e-2	1	NC	5	NC	3
34		min	-.485	4	-.314	1	.006	12	-1.173e-3	3	556.526	3	3000.503	1
35	18	max	0	12	.23	3	.035	1	1.016e-2	1	NC	5	NC	2
36		min	-.485	4	-.126	1	.002	10	-1.126e-3	3	1006.698	3	7360.682	1
37	19	max	0	12	.111	1	.004	3	8.819e-3	1	NC	1	NC	1
38		min	-.485	4	-.014	3	-.001	10	-1.079e-3	3	NC	1	NC	1
39	M14	1	max	0	.147	3	.003	3	5.552e-3	1	NC	1	NC	1
40		min	-.387	4	-.36	1	-.001	10	-2.663e-3	3	NC	1	NC	1
41	2	max	0	1	.381	3	.025	1	6.679e-3	1	NC	5	NC	1
42		min	-.387	4	-.728	1	-.02	5	-3.249e-3	3	667.623	1	NC	1
43	3	max	0	1	.579	3	.068	1	7.805e-3	1	NC	15	NC	3
44		min	-.387	4	-1.044	1	-.024	5	-3.836e-3	3	359.499	1	3733.123	1
45	4	max	0	1	.715	3	.109	1	8.932e-3	1	NC	15	NC	3
46		min	-.387	4	-1.271	1	-.016	5	-4.422e-3	3	269.971	1	2309.742	1
47	5	max	0	1	.777	3	.132	1	1.006e-2	1	9468.875	15	NC	3
48		min	-.387	4	-1.391	1	-.002	5	-5.009e-3	3	238.619	1	1895.28	1
49	6	max	0	1	.764	3	.131	1	1.119e-2	1	9394.133	15	NC	3
50		min	-.387	4	-1.403	1	.009	15	-5.595e-3	3	235.843	1	1914.977	1
51	7	max	0	1	.691	3	.105	1	1.231e-2	1	NC	15	NC	3
52		min	-.387	4	-1.325	1	.006	10	-6.182e-3	3	254.884	1	2392.954	1
53	8	max	0	1	.583	3	.063	1	1.344e-2	1	NC	15	NC	2
54		min	-.387	4	-1.192	1	.002	10	-6.768e-3	3	295.454	1	4050.796	1
55	9	max	0	1	.479	3	.027	4	1.457e-2	1	NC	15	NC	1
56		min	-.387	4	-1.058	1	-.002	10	-7.355e-3	3	352.006	1	8946.295	4
57	10	max	0	1	.43	3	.01	3	1.569e-2	1	NC	5	NC	1
58		min	-.387	4	-.995	1	-.006	2	-7.941e-3	3	387.312	1	NC	1
59	11	max	0	12	.479	3	.02	1	1.457e-2	1	NC	15	NC	1
60		min	-.387	4	-1.058	1	-.02	5	-7.355e-3	3	352.006	1	NC	1
61	12	max	0	12	.583	3	.063	1	1.344e-2	1	NC	15	NC	2
62		min	-.387	4	-1.192	1	-.023	5	-6.768e-3	3	295.454	1	4050.796	1
63	13	max	0	12	.691	3	.105	1	1.231e-2	1	NC	15	NC	3
64		min	-.387	4	-1.325	1	-.014	5	-6.182e-3	3	254.884	1	2392.954	1
65	14	max	0	12	.764	3	.131	1	1.119e-2	1	9393.797	15	NC	3
66		min	-.387	4	-1.403	1	0	15	-5.595e-3	3	235.843	1	1914.977	1
67	15	max	0	12	.777	3	.132	1	1.006e-2	1	9468.447	15	NC	3
68		min	-.387	4	-1.391	1	.008	12	-5.009e-3	3	238.619	1	1895.28	1
69	16	max	0	12	.715	3	.109	1	8.932e-3	1	NC	15	NC	3
70		min	-.387	4	-1.271	1	.007	12	-4.422e-3	3	269.971	1	2309.742	1
71	17	max	0	12	.579	3	.068	1	7.805e-3	1	NC	15	NC	3
72		min	-.387	4	-1.044	1	.005	10	-3.836e-3	3	359.499	1	3733.123	1
73	18	max	0	12	.381	3	.028	4	6.679e-3	1	NC	5	NC	1
74		min	-.387	4	-.728	1	0	10	-3.249e-3	3	667.623	1	8652.582	4
75	19	max	0	12	.147	3	.003	3	5.552e-3	1	NC	1	NC	1
76		min	-.387	4	-.36	1	-.001	10	-2.663e-3	3	NC	1	NC	1
77	M15	1	max	0	.15	3	.003	3	2.244e-3	3	NC	1	NC	1
78		min	-.326	4	-.359	1	0	10	-5.645e-3	1	NC	1	NC	1
79	2	max	0	12	.299	3	.025	1	2.74e-3	3	NC	5	NC	1
80		min	-.326	4	-.761	1	-.03	5	-6.795e-3	1	612.399	1	7921.174	5
81	3	max	0	12	.428	3	.068	1	3.236e-3	3	NC	15	NC	3
82		min	-.326	4	-1.104	1	-.037	5	-7.945e-3	1	330.469	1	3722.999	1
83	4	max	0	12	.522	3	.109	1	3.732e-3	3	NC	15	NC	3
84		min	-.326	4	-1.347	1	-.026	5	-9.095e-3	1	249.089	1	2304.805	1
85	5	max	0	12	.575	3	.132	1	4.229e-3	3	9476.982	15	NC	3
86		min	-.326	4	-1.47	1	-.007	5	-1.024e-2	1	221.42	1	1891.544	1
87	6	max	0	12	.585	3	.131	1	4.725e-3	3	9403.738	15	NC	3
88		min	-.326	4	-1.474	1	.009	12	-1.139e-2	1	220.721	1	1910.98	1
89	7	max	0	12	.56	3	.105	1	5.221e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90			min	-.326	4	-1.377	1	.006	10	-1.254e-2	1	241.594	1	2386.688	1
91		8	max	0	12	.513	3	.063	1	5.717e-3	3	NC	15	NC	2
92			min	-.326	4	-1.222	1	.002	10	-1.37e-2	1	285.243	1	4033.209	1
93		9	max	0	12	.464	3	.036	4	6.214e-3	3	NC	15	NC	1
94			min	-.326	4	-1.067	1	-.002	10	-1.485e-2	1	347.674	1	6741.695	4
95		10	max	0	1	.44	3	.009	3	6.71e-3	3	NC	5	NC	1
96			min	-.326	4	-.994	1	-.006	2	-1.6e-2	1	387.819	1	NC	1
97		11	max	0	1	.464	3	.02	1	6.214e-3	3	NC	15	NC	1
98			min	-.326	4	-1.067	1	-.029	5	-1.485e-2	1	347.674	1	8513.987	5
99		12	max	0	1	.513	3	.063	1	5.717e-3	3	NC	15	NC	2
100			min	-.326	4	-1.222	1	-.034	5	-1.37e-2	1	285.243	1	4033.209	1
101		13	max	0	1	.56	3	.105	1	5.221e-3	3	NC	15	NC	3
102			min	-.326	4	-1.377	1	-.022	5	-1.254e-2	1	241.594	1	2386.688	1
103		14	max	0	1	.585	3	.131	1	4.725e-3	3	9403.492	15	NC	3
104			min	-.326	4	-1.474	1	-.002	5	-1.139e-2	1	220.721	1	1910.98	1
105		15	max	0	1	.575	3	.132	1	4.229e-3	3	9476.671	15	NC	3
106			min	-.326	4	-1.47	1	.008	12	-1.024e-2	1	221.42	1	1891.544	1
107		16	max	0	1	.522	3	.109	1	3.732e-3	3	NC	15	NC	3
108			min	-.326	4	-1.347	1	.006	12	-9.095e-3	1	249.089	1	2304.805	1
109		17	max	0	1	.428	3	.068	1	3.236e-3	3	NC	15	NC	3
110			min	-.326	4	-1.104	1	.005	10	-7.945e-3	1	330.469	1	3722.999	1
111		18	max	0	1	.299	3	.039	4	2.74e-3	3	NC	5	NC	1
112			min	-.326	4	-.761	1	0	10	-6.795e-3	1	612.399	1	6360.204	4
113		19	max	0	1	.15	3	.003	3	2.244e-3	3	NC	1	NC	1
114			min	-.326	4	-.359	1	0	10	-5.645e-3	1	NC	1	NC	1
115	M16	1	max	0	12	.108	1	.003	3	3.896e-3	3	NC	1	NC	1
116			min	-.143	4	-.049	3	0	10	-8.307e-3	1	NC	1	NC	1
117		2	max	0	12	.038	3	.035	1	4.6e-3	3	NC	5	NC	2
118			min	-.144	4	-.164	1	-.023	5	-9.527e-3	1	905.148	1	7404.104	1
119		3	max	0	12	.106	3	.084	1	5.304e-3	3	NC	5	NC	3
120			min	-.144	4	-.38	1	-.028	5	-1.075e-2	1	504.33	1	3008.497	1
121		4	max	0	12	.143	3	.126	1	6.008e-3	3	NC	5	NC	3
122			min	-.144	4	-.503	1	-.021	5	-1.197e-2	1	402.805	1	1982.863	1
123		5	max	0	12	.143	3	.148	1	6.712e-3	3	NC	5	NC	3
124			min	-.144	4	-.516	1	-.007	5	-1.319e-2	1	394.53	1	1683.201	1
125		6	max	0	12	.107	3	.143	1	7.417e-3	3	NC	5	NC	3
126			min	-.144	4	-.422	1	.005	15	-1.441e-2	1	464.751	1	1736.307	1
127		7	max	0	12	.043	3	.114	1	8.121e-3	3	NC	5	NC	3
128			min	-.144	4	-.244	1	.007	10	-1.563e-2	1	700.068	1	2198.153	1
129		8	max	0	12	.004	4	.067	1	8.825e-3	3	NC	3	NC	2
130			min	-.144	4	-.051	2	.003	10	-1.685e-2	1	1835.44	2	3743.435	1
131		9	max	0	12	.169	1	.026	4	9.529e-3	3	NC	4	NC	1
132			min	-.144	4	-.102	3	-.001	10	-1.807e-2	1	4007.657	1	9457.215	4
133		10	max	0	1	.256	1	.008	3	1.023e-2	3	NC	5	NC	1
134			min	-.144	4	-.132	3	-.005	2	-1.929e-2	1	1654.85	1	NC	1
135		11	max	0	1	.169	1	.021	1	9.529e-3	3	NC	4	NC	1
136			min	-.144	4	-.102	3	-.018	5	-1.807e-2	1	4007.657	1	NC	1
137		12	max	0	1	.004	6	.067	1	8.825e-3	3	NC	3	NC	2
138			min	-.143	4	-.051	2	-.019	5	-1.685e-2	1	1835.44	2	3743.435	1
139		13	max	0	1	.043	3	.114	1	8.121e-3	3	NC	5	NC	3
140			min	-.143	4	-.244	1	-.009	5	-1.563e-2	1	700.068	1	2198.153	1
141		14	max	0	1	.107	3	.143	1	7.417e-3	3	NC	5	NC	3
142			min	-.143	4	-.422	1	.004	15	-1.441e-2	1	464.751	1	1736.307	1
143		15	max	0	1	.143	3	.148	1	6.712e-3	3	NC	5	NC	3
144			min	-.143	4	-.516	1	.008	12	-1.319e-2	1	394.53	1	1683.201	1
145		16	max	0	1	.143	3	.126	1	6.008e-3	3	NC	5	NC	3
146			min	-.143	4	-.503	1	.007	12	-1.197e-2	1	402.805	1	1982.863	1



Company : Schletter, Inc.  
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Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.106	3	.084	1	5.304e-3	3	NC	5	NC	3
148			min	-.143	4	-.38	1	.005	12	-1.075e-2	1	504.33	1	3008.497	1
149		18	max	0	1	.038	3	.035	1	4.6e-3	3	NC	5	NC	2
150			min	-.143	4	-.164	1	.002	10	-9.527e-3	1	905.148	1	7224.676	4
151		19	max	0	1	.108	1	.003	3	3.896e-3	3	NC	1	NC	1
152			min	-.143	4	-.049	3	0	10	-8.307e-3	1	NC	1	NC	1
153	M2	1	max	.005	1	.002	2	.007	1	1.213e-3	5	NC	1	NC	2
154			min	-.004	3	-.005	3	-.459	4	-1.756e-4	1	NC	1	104.32	4
155		2	max	.005	1	.002	2	.006	1	1.3e-3	5	NC	1	NC	2
156			min	-.004	3	-.005	3	-.421	4	-1.627e-4	1	NC	1	113.67	4
157		3	max	.005	1	.001	2	.006	1	1.387e-3	5	NC	1	NC	2
158			min	-.003	3	-.004	3	-.383	4	-1.498e-4	1	NC	1	124.789	4
159		4	max	.004	1	0	2	.005	1	1.473e-3	5	NC	1	NC	2
160			min	-.003	3	-.004	3	-.346	4	-1.369e-4	1	NC	1	138.138	4
161		5	max	.004	1	0	2	.005	1	1.56e-3	5	NC	1	NC	1
162			min	-.003	3	-.004	3	-.31	4	-1.24e-4	1	NC	1	154.349	4
163		6	max	.004	1	0	2	.004	1	1.647e-3	5	NC	1	NC	1
164			min	-.003	3	-.004	3	-.275	4	-1.112e-4	1	NC	1	174.296	4
165		7	max	.003	1	0	15	.004	1	1.733e-3	5	NC	1	NC	1
166			min	-.003	3	-.004	3	-.24	4	-9.826e-5	1	NC	1	199.222	4
167		8	max	.003	1	0	15	.003	1	1.82e-3	4	NC	1	NC	1
168			min	-.002	3	-.004	3	-.207	4	-8.537e-5	1	NC	1	230.944	4
169		9	max	.003	1	0	15	.003	1	1.911e-3	4	NC	1	NC	1
170			min	-.002	3	-.004	3	-.176	4	-7.248e-5	1	NC	1	272.202	4
171		10	max	.003	1	0	15	.002	1	2.003e-3	4	NC	1	NC	1
172			min	-.002	3	-.003	3	-.146	4	-5.959e-5	1	NC	1	327.288	4
173		11	max	.002	1	0	15	.002	1	2.094e-3	4	NC	1	NC	1
174			min	-.002	3	-.003	3	-.119	4	-4.67e-5	1	NC	1	403.247	4
175		12	max	.002	1	0	15	.001	1	2.186e-3	4	NC	1	NC	1
176			min	-.002	3	-.003	3	-.093	4	-3.38e-5	1	NC	1	512.307	4
177		13	max	.002	1	0	15	.001	1	2.277e-3	4	NC	1	NC	1
178			min	-.001	3	-.003	3	-.071	4	-2.091e-5	1	NC	1	677.265	4
179		14	max	.001	1	0	15	0	1	2.368e-3	4	NC	1	NC	1
180			min	-.001	3	-.002	3	-.051	4	-8.022e-6	1	NC	1	944.776	4
181		15	max	.001	1	0	15	0	1	2.46e-3	4	NC	1	NC	1
182			min	0	3	-.002	3	-.034	4	0	3	NC	1	1423.249	4
183		16	max	0	1	0	15	0	1	2.551e-3	4	NC	1	NC	1
184			min	0	3	-.001	6	-.02	4	5.06e-7	12	NC	1	2415.887	4
185		17	max	0	1	0	15	0	1	2.643e-3	4	NC	1	NC	1
186			min	0	3	-.001	6	-.009	4	1.019e-6	12	NC	1	5068.432	4
187		18	max	0	1	0	15	0	1	2.734e-3	4	NC	1	NC	1
188			min	0	3	0	6	-.003	4	1.532e-6	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.825e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	2.046e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-6.397e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.299e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.014	4	4.24e-6	1	NC	1	NC	1
194			min	0	2	-.001	6	0	12	-4.581e-6	5	NC	1	NC	1
195		3	max	0	3	0	15	.027	4	6.251e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	9.69e-7	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.039	4	1.253e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	1.773e-6	12	NC	1	NC	1
199		5	max	0	3	-.001	15	.051	4	1.88e-3	4	NC	1	NC	1
200			min	0	2	-.007	6	0	12	2.578e-6	12	NC	1	8714.412	4
201		6	max	0	3	-.002	15	.063	4	2.508e-3	4	NC	1	NC	1
202			min	0	2	-.008	6	0	12	3.382e-6	12	NC	1	7842.706	4
203		7	max	0	3	-.002	15	.073	4	3.135e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	0	2	-.01	6	0	12	4.186e-6	12	9458.294	6	7448.54	4
205		8	max	.001	3	-.002	15	.084	4	3.763e-3	4	NC	1	NC	1
206			min	0	2	-.011	6	0	12	4.991e-6	12	8418.843	6	7386.754	4
207		9	max	.001	3	-.003	15	.094	4	4.39e-3	4	NC	1	NC	1
208			min	0	2	-.012	6	0	12	5.795e-6	12	7796.615	6	7607.453	5
209		10	max	.001	3	-.003	15	.103	4	5.018e-3	4	NC	2	NC	1
210			min	-.001	2	-.012	6	0	12	6.599e-6	12	7480.521	6	8124.71	5
211		11	max	.002	3	-.003	15	.112	4	5.645e-3	4	NC	2	NC	1
212			min	-.001	2	-.013	6	0	12	7.404e-6	12	7422.962	6	9016.007	5
213		12	max	.002	3	-.003	15	.121	4	6.273e-3	4	NC	2	NC	1
214			min	-.001	2	-.012	6	0	12	8.208e-6	12	7621.395	6	NC	1
215		13	max	.002	3	-.002	15	.13	4	6.9e-3	4	NC	1	NC	1
216			min	-.001	2	-.012	6	0	12	9.012e-6	12	8119.561	6	NC	1
217		14	max	.002	3	-.002	15	.138	4	7.528e-3	4	NC	1	NC	1
218			min	-.001	2	-.01	6	0	12	9.817e-6	12	9030.954	6	NC	1
219		15	max	.002	3	-.002	15	.147	4	8.155e-3	4	NC	1	NC	1
220			min	-.002	2	-.009	1	0	12	1.062e-5	12	NC	1	NC	1
221		16	max	.002	3	-.001	15	.155	4	8.783e-3	4	NC	1	NC	1
222			min	-.002	2	-.008	1	0	12	1.143e-5	12	NC	1	NC	1
223		17	max	.003	3	0	15	.164	4	9.411e-3	4	NC	1	NC	1
224			min	-.002	2	-.006	1	0	12	1.223e-5	12	NC	1	NC	1
225		18	max	.003	3	0	15	.174	4	1.004e-2	4	NC	1	NC	1
226			min	-.002	2	-.005	1	0	12	1.303e-5	12	NC	1	NC	1
227		19	max	.003	3	0	5	.184	4	1.067e-2	4	NC	1	NC	1
228			min	-.002	2	-.003	1	0	12	1.384e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	3
230			min	0	3	-.003	3	-.184	4	-9.493e-4	4	NC	1	134.945	4
231		2	max	.003	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	2
232			min	0	3	-.003	3	-.169	4	-9.493e-4	4	NC	1	146.926	4
233		3	max	.003	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	2
234			min	0	3	-.003	3	-.154	4	-9.493e-4	4	NC	1	161.173	4
235		4	max	.002	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	2
236			min	0	3	-.002	3	-.139	4	-9.493e-4	4	NC	1	178.278	4
237		5	max	.002	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	2
238			min	0	3	-.002	3	-.125	4	-9.493e-4	4	NC	1	199.043	4
239		6	max	.002	1	.001	2	0	12	-3.986e-7	12	NC	1	NC	2
240			min	0	3	-.002	3	-.11	4	-9.493e-4	4	NC	1	224.581	4
241		7	max	.002	1	0	2	0	12	-3.986e-7	12	NC	1	NC	2
242			min	0	3	-.002	3	-.097	4	-9.493e-4	4	NC	1	256.473	4
243		8	max	.002	1	0	2	0	12	-3.986e-7	12	NC	1	NC	2
244			min	0	3	-.002	3	-.084	4	-9.493e-4	4	NC	1	297.022	4
245		9	max	.002	1	0	2	0	12	-3.986e-7	12	NC	1	NC	2
246			min	0	3	-.002	3	-.071	4	-9.493e-4	4	NC	1	349.699	4
247		10	max	.001	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
248			min	0	3	-.001	3	-.059	4	-9.493e-4	4	NC	1	419.93	4
249		11	max	.001	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
250			min	0	3	-.001	3	-.048	4	-9.493e-4	4	NC	1	516.589	4
251		12	max	.001	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
252			min	0	3	-.001	3	-.038	4	-9.493e-4	4	NC	1	655.032	4
253		13	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
254			min	0	3	0	3	-.029	4	-9.493e-4	4	NC	1	863.768	4
255		14	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
256			min	0	3	0	3	-.021	4	-9.493e-4	4	NC	1	1200.803	4
257		15	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
258			min	0	3	0	3	-.014	4	-9.493e-4	4	NC	1	1799.882	4
259		16	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
260			min	0	3	0	3	-.008	4	-9.493e-4	4	NC	1	3030.836	4



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-9.493e-4	4	NC	1	6265.463	4
263		18	max	0	1	0	2	0	12	-3.986e-7	12	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-9.493e-4	4	NC	1	NC	1
265		19	max	0	1	0	1	0	1	-3.986e-7	12	NC	1	NC	1
266			min	0	1	0	1	0	1	-9.493e-4	4	NC	1	NC	1
267	M6	1	max	.016	1	.01	2	0	1	1.267e-3	4	NC	3	NC	1
268			min	-.013	3	-.014	3	-.463	4	0	1	4842.218	2	103.432	4
269		2	max	.016	1	.009	2	0	1	1.352e-3	4	NC	3	NC	1
270			min	-.012	3	-.014	3	-.425	4	0	1	5342.275	2	112.703	4
271		3	max	.015	1	.008	2	0	1	1.438e-3	4	NC	1	NC	1
272			min	-.011	3	-.013	3	-.387	4	0	1	5952.237	2	123.728	4
273		4	max	.014	1	.007	2	0	1	1.523e-3	4	NC	1	NC	1
274			min	-.011	3	-.012	3	-.349	4	0	1	6706.069	2	136.965	4
275		5	max	.013	1	.006	2	0	1	1.609e-3	4	NC	1	NC	1
276			min	-.01	3	-.011	3	-.313	4	0	1	7652.437	2	153.04	4
277		6	max	.012	1	.005	2	0	1	1.694e-3	4	NC	1	NC	1
278			min	-.009	3	-.011	3	-.277	4	0	1	8863.148	2	172.82	4
279		7	max	.011	1	.005	2	0	1	1.78e-3	4	NC	1	NC	1
280			min	-.009	3	-.01	3	-.242	4	0	1	NC	1	197.538	4
281		8	max	.01	1	.004	2	0	1	1.865e-3	4	NC	1	NC	1
282			min	-.008	3	-.009	3	-.209	4	0	1	NC	1	228.996	4
283		9	max	.009	1	.003	2	0	1	1.951e-3	4	NC	1	NC	1
284			min	-.007	3	-.008	3	-.177	4	0	1	NC	1	269.911	4
285		10	max	.008	1	.002	2	0	1	2.036e-3	4	NC	1	NC	1
286			min	-.006	3	-.008	3	-.147	4	0	1	NC	1	324.541	4
287		11	max	.007	1	.002	2	0	1	2.121e-3	4	NC	1	NC	1
288			min	-.006	3	-.007	3	-.12	4	0	1	NC	1	399.874	4
289		12	max	.006	1	.001	2	0	1	2.207e-3	4	NC	1	NC	1
290			min	-.005	3	-.006	3	-.094	4	0	1	NC	1	508.039	4
291		13	max	.005	1	0	2	0	1	2.292e-3	4	NC	1	NC	1
292			min	-.004	3	-.005	3	-.071	4	0	1	NC	1	671.655	4
293		14	max	.005	1	0	2	0	1	2.378e-3	4	NC	1	NC	1
294			min	-.004	3	-.004	3	-.051	4	0	1	NC	1	937.009	4
295		15	max	.004	1	0	2	0	1	2.463e-3	4	NC	1	NC	1
296			min	-.003	3	-.004	3	-.034	4	0	1	NC	1	1411.678	4
297		16	max	.003	1	0	2	0	1	2.549e-3	4	NC	1	NC	1
298			min	-.002	3	-.003	3	-.02	4	0	1	NC	1	2396.591	4
299		17	max	.002	1	0	2	0	1	2.634e-3	4	NC	1	NC	1
300			min	-.001	3	-.002	3	-.01	4	0	1	NC	1	5029.276	4
301		18	max	0	1	0	2	0	1	2.72e-3	4	NC	1	NC	1
302			min	0	3	0	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.805e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-6.231e-4	4	NC	1	NC	1
307		2	max	0	3	0	15	.014	4	0	1	NC	1	NC	1
308			min	0	2	-.002	3	0	1	-9.191e-6	5	NC	1	NC	1
309		3	max	.001	3	0	15	.027	4	6.069e-4	4	NC	1	NC	1
310			min	0	2	-.004	3	0	1	0	1	NC	1	NC	1
311		4	max	.002	3	-.001	15	.039	4	1.222e-3	4	NC	1	NC	1
312			min	-.001	2	-.005	3	0	1	0	1	NC	1	NC	1
313		5	max	.002	3	-.002	15	.051	4	1.837e-3	4	NC	1	NC	1
314			min	-.002	2	-.007	4	0	1	0	1	NC	1	8359.493	4
315		6	max	.003	3	-.002	15	.062	4	2.452e-3	4	NC	1	NC	1
316			min	-.002	2	-.009	4	0	1	0	1	NC	1	7499.671	4
317		7	max	.003	3	-.002	15	.073	4	3.067e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.003	2	-.01	4	0	1	0	1	9561.6	4	7096.68	4
319	8	max	.004	3	-.003	15	.083	4	3.682e-3	4	NC	1	NC	1
320		min	-.003	2	-.011	4	0	1	0	1	8503.383	4	7007.413	4
321	9	max	.004	3	-.003	15	.093	4	4.297e-3	4	NC	1	NC	1
322		min	-.004	2	-.012	4	0	1	0	1	7869.295	4	7183.674	4
323	10	max	.005	3	-.003	15	.102	4	4.912e-3	4	NC	1	NC	1
324		min	-.004	2	-.013	4	0	1	0	1	7545.805	4	7633.547	4
325	11	max	.005	3	-.003	15	.111	4	5.527e-3	4	NC	1	NC	1
326		min	-.005	2	-.013	4	0	1	0	1	7484.071	4	8417.327	4
327	12	max	.006	3	-.003	15	.119	4	6.142e-3	4	NC	1	NC	1
328		min	-.005	2	-.013	4	0	1	0	1	7680.99	4	9668.169	4
329	13	max	.006	3	-.003	15	.128	4	6.757e-3	4	NC	1	NC	1
330		min	-.006	2	-.012	4	0	1	0	1	8180.259	4	NC	1
331	14	max	.007	3	-.003	15	.136	4	7.372e-3	4	NC	1	NC	1
332		min	-.006	2	-.012	1	0	1	0	1	9095.899	4	NC	1
333	15	max	.007	3	-.002	15	.144	4	7.987e-3	4	NC	1	NC	1
334		min	-.007	2	-.011	1	0	1	0	1	NC	1	NC	1
335	16	max	.008	3	-.002	15	.153	4	8.602e-3	4	NC	1	NC	1
336		min	-.007	2	-.011	1	0	1	0	1	NC	1	NC	1
337	17	max	.008	3	-.001	15	.161	4	9.217e-3	4	NC	1	NC	1
338		min	-.008	2	-.01	1	0	1	0	1	NC	1	NC	1
339	18	max	.009	3	0	15	.17	4	9.832e-3	4	NC	1	NC	1
340		min	-.008	2	-.009	1	0	1	0	1	NC	1	NC	1
341	19	max	.009	3	0	15	.18	4	1.045e-2	4	NC	1	NC	1
342		min	-.009	2	-.007	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.009	1	.007	2	0	0	1	NC	1	NC	1
344		min	-.002	3	-.009	3	-.18	4	-9.711e-4	4	NC	1	137.948	4
345	2	max	.008	1	.007	2	0	1	0	1	NC	1	NC	1
346		min	-.002	3	-.009	3	-.165	4	-9.711e-4	4	NC	1	150.197	4
347	3	max	.008	1	.006	2	0	1	0	1	NC	1	NC	1
348		min	-.002	3	-.008	3	-.151	4	-9.711e-4	4	NC	1	164.764	4
349	4	max	.007	1	.006	2	0	1	0	1	NC	1	NC	1
350		min	-.002	3	-.008	3	-.136	4	-9.711e-4	4	NC	1	182.252	4
351	5	max	.007	1	.006	2	0	1	0	1	NC	1	NC	1
352		min	-.002	3	-.007	3	-.122	4	-9.711e-4	4	NC	1	203.482	4
353	6	max	.006	1	.005	2	0	1	0	1	NC	1	NC	1
354		min	-.001	3	-.007	3	-.108	4	-9.711e-4	4	NC	1	229.593	4
355	7	max	.006	1	.005	2	0	1	0	1	NC	1	NC	1
356		min	-.001	3	-.006	3	-.095	4	-9.711e-4	4	NC	1	262.199	4
357	8	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
358		min	-.001	3	-.006	3	-.082	4	-9.711e-4	4	NC	1	303.656	4
359	9	max	.005	1	.004	2	0	1	0	1	NC	1	NC	1
360		min	-.001	3	-.005	3	-.069	4	-9.711e-4	4	NC	1	357.514	4
361	10	max	.004	1	.004	2	0	1	0	1	NC	1	NC	1
362		min	-.001	3	-.005	3	-.058	4	-9.711e-4	4	NC	1	429.317	4
363	11	max	.004	1	.003	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.004	3	-.047	4	-9.711e-4	4	NC	1	528.141	4
365	12	max	.003	1	.003	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.004	3	-.037	4	-9.711e-4	4	NC	1	669.687	4
367	13	max	.003	1	.002	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.003	3	-.028	4	-9.711e-4	4	NC	1	883.099	4
369	14	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.003	3	-.02	4	-9.711e-4	4	NC	1	1227.686	4
371	15	max	.002	1	.002	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.002	3	-.013	4	-9.711e-4	4	NC	1	1840.191	4
373	16	max	.001	1	.001	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.002	3	-.008	4	-9.711e-4	4	NC	1	3098.739	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	0	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.001	3	-.004	4	-9.711e-4	4	NC	1	6405.897	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	0	3	-.001	4	-9.711e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-9.711e-4	4	NC	1	NC	1
381	M10	1	max	.005	1	.002	2	0	12	1.268e-3	4	NC	1	NC	2
382			min	-.004	3	-.005	3	-.462	4	7.192e-6	12	NC	1	103.55	4
383		2	max	.005	1	.002	2	0	12	1.353e-3	4	NC	1	NC	2
384			min	-.004	3	-.005	3	-.424	4	6.679e-6	12	NC	1	112.832	4
385		3	max	.005	1	.001	2	0	12	1.438e-3	4	NC	1	NC	2
386			min	-.003	3	-.004	3	-.386	4	6.165e-6	12	NC	1	123.869	4
387		4	max	.004	1	0	2	0	12	1.523e-3	4	NC	1	NC	2
388			min	-.003	3	-.004	3	-.349	4	5.652e-6	12	NC	1	137.121	4
389		5	max	.004	1	0	2	0	12	1.608e-3	4	NC	1	NC	1
390			min	-.003	3	-.004	3	-.312	4	5.139e-6	12	NC	1	153.215	4
391		6	max	.004	1	0	2	0	12	1.693e-3	4	NC	1	NC	1
392			min	-.003	3	-.004	3	-.277	4	4.626e-6	12	NC	1	173.017	4
393		7	max	.003	1	0	2	0	12	1.778e-3	4	NC	1	NC	1
394			min	-.003	3	-.004	3	-.242	4	4.113e-6	12	NC	1	197.764	4
395		8	max	.003	1	0	10	0	12	1.863e-3	4	NC	1	NC	1
396			min	-.002	3	-.004	3	-.209	4	3.599e-6	12	NC	1	229.257	4
397		9	max	.003	1	0	10	0	12	1.948e-3	4	NC	1	NC	1
398			min	-.002	3	-.004	3	-.177	4	3.086e-6	12	NC	1	270.219	4
399		10	max	.003	1	0	10	0	12	2.033e-3	4	NC	1	NC	1
400			min	-.002	3	-.003	3	-.147	4	2.573e-6	12	NC	1	324.912	4
401		11	max	.002	1	0	15	0	12	2.118e-3	4	NC	1	NC	1
402			min	-.002	3	-.003	3	-.12	4	2.06e-6	12	NC	1	400.332	4
403		12	max	.002	1	0	15	0	12	2.202e-3	4	NC	1	NC	1
404			min	-.002	3	-.003	3	-.094	4	1.547e-6	12	NC	1	508.623	4
405		13	max	.002	1	0	15	0	12	2.287e-3	4	NC	1	NC	1
406			min	-.001	3	-.003	4	-.071	4	1.034e-6	12	NC	1	672.429	4
407		14	max	.001	1	0	15	0	12	2.372e-3	4	NC	1	NC	1
408			min	-.001	3	-.002	4	-.051	4	5.204e-7	12	NC	1	938.094	4
409		15	max	.001	1	0	15	0	12	2.457e-3	4	NC	1	NC	1
410			min	0	3	-.002	4	-.034	4	-4.869e-6	1	NC	1	1413.325	4
411		16	max	0	1	0	15	0	12	2.542e-3	4	NC	1	NC	1
412			min	0	3	-.002	4	-.02	4	-1.776e-5	1	NC	1	2399.421	4
413		17	max	0	1	0	15	0	12	2.627e-3	4	NC	1	NC	1
414			min	0	3	-.001	4	-.01	4	-3.065e-5	1	NC	1	5035.356	4
415		18	max	0	1	0	15	0	12	2.712e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.003	4	-4.354e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.797e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-5.643e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.749e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.211e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.014	4	-1.646e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-4.992e-6	5	NC	1	NC	1
423		3	max	0	3	0	15	.027	4	6.121e-4	4	NC	1	NC	1
424			min	0	2	-.003	4	0	1	-2.597e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.039	4	1.229e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-4.77e-5	1	NC	1	NC	1
427		5	max	0	3	-.002	15	.051	4	1.845e-3	4	NC	1	NC	1
428			min	0	2	-.007	4	-.001	1	-6.944e-5	1	NC	1	8596.943	4
429		6	max	0	3	-.002	15	.062	4	2.462e-3	4	NC	1	NC	1
430			min	0	2	-.009	4	-.001	1	-9.117e-5	1	NC	1	7731.557	4
431		7	max	0	3	-.003	15	.073	4	3.078e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	0	2	-.01	4	-.002	1	-1.129e-4	1	9108.365	4	7336.789	4
433		8	max	.001	3	-.003	15	.083	4	3.695e-3	4	NC	1	NC	1
434			min	0	2	-.012	4	-.002	1	-1.346e-4	1	8131.383	4	7268.506	4
435		9	max	.001	3	-.003	15	.092	4	4.312e-3	4	NC	1	NC	1
436			min	0	2	-.013	4	-.002	1	-1.564e-4	1	7548.702	4	7480.733	4
437		10	max	.001	3	-.003	15	.102	4	4.928e-3	4	NC	2	NC	1
438			min	-.001	2	-.013	4	-.003	1	-1.781e-4	1	7257.253	4	7987.263	4
439		11	max	.002	3	-.003	15	.111	4	5.545e-3	4	NC	2	NC	1
440			min	-.001	2	-.013	4	-.003	1	-1.998e-4	1	7213.522	4	8859.68	4
441		12	max	.002	3	-.003	15	.119	4	6.161e-3	4	NC	2	NC	1
442			min	-.001	2	-.013	4	-.004	1	-2.216e-4	1	7416.773	4	NC	1
443		13	max	.002	3	-.003	15	.128	4	6.778e-3	4	NC	1	NC	1
444			min	-.001	2	-.012	4	-.004	1	-2.433e-4	1	7910.838	4	NC	1
445		14	max	.002	3	-.003	15	.136	4	7.395e-3	4	NC	1	NC	1
446			min	-.001	2	-.011	4	-.004	1	-2.65e-4	1	8807.343	4	NC	1
447		15	max	.002	3	-.002	15	.144	4	8.011e-3	4	NC	1	NC	1
448			min	-.002	2	-.01	4	-.005	1	-2.868e-4	1	NC	1	NC	1
449		16	max	.002	3	-.002	15	.153	4	8.628e-3	4	NC	1	NC	1
450			min	-.002	2	-.008	4	-.005	1	-3.085e-4	1	NC	1	NC	1
451		17	max	.003	3	-.001	15	.162	4	9.244e-3	4	NC	1	NC	1
452			min	-.002	2	-.006	1	-.006	1	-3.302e-4	1	NC	1	NC	1
453		18	max	.003	3	0	15	.171	4	9.861e-3	4	NC	1	NC	1
454			min	-.002	2	-.005	1	-.006	1	-3.52e-4	1	NC	1	NC	1
455		19	max	.003	3	0	12	.181	4	1.048e-2	4	NC	1	NC	1
456			min	-.002	2	-.003	1	-.006	1	-3.737e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.001	2	.006	1	1.369e-5	1	NC	1	NC	3
458			min	0	3	-.003	3	-.181	4	-9.423e-4	4	NC	1	137.388	4
459		2	max	.003	1	.001	2	.006	1	1.369e-5	1	NC	1	NC	2
460			min	0	3	-.003	3	-.166	4	-9.423e-4	4	NC	1	149.585	4
461		3	max	.003	1	.001	2	.005	1	1.369e-5	1	NC	1	NC	2
462			min	0	3	-.003	3	-.151	4	-9.423e-4	4	NC	1	164.089	4
463		4	max	.002	1	.001	2	.005	1	1.369e-5	1	NC	1	NC	2
464			min	0	3	-.002	3	-.137	4	-9.423e-4	4	NC	1	181.502	4
465		5	max	.002	1	.001	2	.004	1	1.369e-5	1	NC	1	NC	2
466			min	0	3	-.002	3	-.122	4	-9.423e-4	4	NC	1	202.641	4
467		6	max	.002	1	.001	2	.004	1	1.369e-5	1	NC	1	NC	2
468			min	0	3	-.002	3	-.108	4	-9.423e-4	4	NC	1	228.639	4
469		7	max	.002	1	0	2	.003	1	1.369e-5	1	NC	1	NC	2
470			min	0	3	-.002	3	-.095	4	-9.423e-4	4	NC	1	261.105	4
471		8	max	.002	1	0	2	.003	1	1.369e-5	1	NC	1	NC	2
472			min	0	3	-.002	3	-.082	4	-9.423e-4	4	NC	1	302.385	4
473		9	max	.002	1	0	2	.003	1	1.369e-5	1	NC	1	NC	2
474			min	0	3	-.002	3	-.07	4	-9.423e-4	4	NC	1	356.011	4
475		10	max	.001	1	0	2	.002	1	1.369e-5	1	NC	1	NC	1
476			min	0	3	-.001	3	-.058	4	-9.423e-4	4	NC	1	427.506	4
477		11	max	.001	1	0	2	.002	1	1.369e-5	1	NC	1	NC	1
478			min	0	3	-.001	3	-.047	4	-9.423e-4	4	NC	1	525.906	4
479		12	max	.001	1	0	2	.001	1	1.369e-5	1	NC	1	NC	1
480			min	0	3	-.001	3	-.037	4	-9.423e-4	4	NC	1	666.843	4
481		13	max	0	1	0	2	.001	1	1.369e-5	1	NC	1	NC	1
482			min	0	3	0	3	-.028	4	-9.423e-4	4	NC	1	879.337	4
483		14	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
484			min	0	3	0	3	-.02	4	-9.423e-4	4	NC	1	1222.439	4
485		15	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
486			min	0	3	0	3	-.014	4	-9.423e-4	4	NC	1	1832.301	4
487		16	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
488			min	0	3	0	3	-.008	4	-9.423e-4	4	NC	1	3085.409	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
490		min	0	3	0	3	-.004	4	-9.423e-4	4	NC	1	6378.232	4
491	18	max	0	1	0	2	0	1	1.369e-5	1	NC	1	NC	1
492		min	0	3	0	3	-.001	4	-9.423e-4	4	NC	1	NC	1
493	19	max	0	1	0	1	0	1	1.369e-5	1	NC	1	NC	1
494		min	0	1	0	1	0	1	-9.423e-4	4	NC	1	NC	1
495	M1	1	max	.004	3	.111	.485	4	1.808e-2	1	NC	1	NC	1
496		min	-.001	10	-.014	3	0	12	-1.855e-2	3	NC	1	NC	1
497	2	max	.004	3	.055	1	.471	4	8.905e-3	4	NC	3	NC	1
498		min	-.001	10	-.007	3	-.005	1	-9.177e-3	3	2036.645	1	NC	1
499	3	max	.004	3	.005	3	.458	4	1.405e-2	4	NC	5	NC	1
500		min	-.001	10	-.006	1	-.007	1	-1.285e-4	1	973.648	1	8625.032	5
501	4	max	.004	3	.027	3	.446	4	1.245e-2	4	NC	5	NC	1
502		min	-.001	10	-.077	1	-.006	1	-3.354e-3	3	607.679	1	5845.637	5
503	5	max	.004	3	.055	3	.433	4	1.084e-2	4	NC	15	NC	1
504		min	-.001	10	-.152	1	-.004	1	-6.618e-3	3	434.407	1	4448.941	5
505	6	max	.003	3	.085	3	.421	4	1.534e-2	1	NC	15	NC	1
506		min	-.001	10	-.225	1	-.002	1	-9.882e-3	3	339.645	1	3632.01	5
507	7	max	.003	3	.114	3	.407	4	2.05e-2	1	9557.39	15	NC	1
508		min	-.001	10	-.291	1	0	12	-1.315e-2	3	284.044	1	3108.69	4
509	8	max	.003	3	.139	3	.393	4	2.565e-2	1	8489.959	15	NC	1
510		min	-.001	10	-.344	1	0	12	-1.641e-2	3	251.3	1	2758.45	4
511	9	max	.003	3	.155	3	.377	4	2.818e-2	1	7933.583	15	NC	1
512		min	-.001	10	-.377	1	0	1	-1.649e-2	3	234.308	1	2563.227	4
513	10	max	.003	3	.161	3	.359	4	2.897e-2	1	7764.188	15	NC	1
514		min	-.001	10	-.388	1	0	12	-1.446e-2	3	229.215	1	2511.656	4
515	11	max	.003	3	.157	3	.339	4	2.976e-2	1	7933.432	15	NC	1
516		min	0	10	-.377	1	0	12	-1.243e-2	3	234.564	1	2580.533	4
517	12	max	.003	3	.144	3	.317	4	2.803e-2	1	8489.611	15	NC	1
518		min	0	10	-.343	1	0	1	-1.038e-2	3	252.096	1	2787.893	5
519	13	max	.003	3	.122	3	.292	4	2.252e-2	1	9556.721	15	NC	1
520		min	0	10	-.29	1	0	1	-8.307e-3	3	286.012	1	3291.407	4
521	14	max	.003	3	.095	3	.264	4	1.702e-2	1	NC	15	NC	1
522		min	0	10	-.223	1	0	12	-6.237e-3	3	343.886	1	4305.05	4
523	15	max	.003	3	.064	3	.237	4	1.151e-2	1	NC	15	NC	1
524		min	0	10	-.149	1	0	12	-4.167e-3	3	443.179	1	6449.133	4
525	16	max	.003	3	.032	3	.209	4	9.293e-3	4	NC	5	NC	1
526		min	0	10	-.074	1	0	12	-2.097e-3	3	626.226	1	NC	1
527	17	max	.003	3	.002	3	.184	4	1.018e-2	4	NC	5	NC	1
528		min	0	10	-.004	1	0	12	-2.651e-5	3	1015.758	1	NC	1
529	18	max	.003	3	.055	1	.162	4	1.041e-2	1	NC	5	NC	1
530		min	0	10	-.025	3	0	12	-3.283e-3	3	2143.851	1	NC	1
531	19	max	.003	3	.108	1	.143	4	2.069e-2	1	NC	1	NC	1
532		min	0	10	-.049	3	0	1	-6.664e-3	3	NC	1	NC	1
533	M5	1	max	.011	3	.266	.484	4	0	1	NC	1	NC	1
534		min	-.007	2	-.022	3	0	1	-2.163e-6	4	NC	1	NC	1
535	2	max	.011	3	.132	1	.474	4	7.196e-3	4	NC	5	NC	1
536		min	-.007	2	-.011	3	0	1	0	1	852.394	1	NC	1
537	3	max	.011	3	.017	3	.462	4	1.417e-2	4	NC	15	NC	1
538		min	-.007	2	-.021	1	0	1	0	1	398.217	1	7211.62	4
539	4	max	.011	3	.076	3	.449	4	1.154e-2	4	9110.921	15	NC	1
540		min	-.007	2	-.208	1	0	1	0	1	241.433	1	5216.18	4
541	5	max	.011	3	.156	3	.436	4	8.92e-3	4	6377.323	15	NC	1
542		min	-.007	2	-.412	1	0	1	0	1	168.652	1	4191.448	4
543	6	max	.011	3	.246	3	.422	4	6.296e-3	4	4910.763	15	NC	1
544		min	-.006	2	-.616	1	0	1	0	1	129.64	1	3560.891	4
545	7	max	.01	3	.334	3	.407	4	3.671e-3	4	4063.65	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.006	2	-.801	1	0	1	0	1	107.121	1	3123.835	4
547		8	max	.01	3	.407	3	.393	4	1.046e-3	4	3571.019	15	NC	1
548			min	-.006	2	-.95	1	0	1	0	1	94.033	1	2791.369	4
549		9	max	.01	3	.455	3	.377	4	0	1	3318.355	15	NC	1
550			min	-.006	2	-1.043	1	0	1	-1.238e-6	5	87.327	1	2563.742	4
551		10	max	.01	3	.472	3	.359	4	0	1	3242.219	15	NC	1
552			min	-.006	2	-1.074	1	0	1	-1.171e-6	5	85.328	1	2529.548	4
553		11	max	.01	3	.46	3	.339	4	0	1	3318.407	15	NC	1
554			min	-.006	2	-1.043	1	0	1	-1.104e-6	5	87.429	1	2605.833	4
555		12	max	.009	3	.421	3	.318	4	7.33e-4	4	3571.146	15	NC	1
556			min	-.006	2	-.948	1	0	1	0	1	94.371	1	2751.208	4
557		13	max	.009	3	.356	3	.292	4	2.572e-3	4	4063.911	15	NC	1
558			min	-.006	2	-.796	1	0	1	0	1	107.999	1	3242.152	4
559		14	max	.009	3	.275	3	.264	4	4.411e-3	4	4911.28	15	NC	1
560			min	-.005	2	-.607	1	0	1	0	1	131.616	1	4423.371	4
561		15	max	.009	3	.184	3	.235	4	6.25e-3	4	6378.354	15	NC	1
562			min	-.005	2	-.401	1	0	1	0	1	172.933	1	7463.8	5
563		16	max	.008	3	.092	3	.206	4	8.089e-3	4	9113.089	15	NC	1
564			min	-.005	2	-.196	1	0	1	0	1	251.007	1	NC	1
565		17	max	.008	3	.006	3	.18	4	9.929e-3	4	NC	15	NC	1
566			min	-.005	2	-.013	1	0	1	0	1	421.459	1	NC	1
567		18	max	.008	3	.132	1	.16	4	5.042e-3	4	NC	5	NC	1
568			min	-.005	2	-.067	3	0	1	0	1	914.571	1	NC	1
569		19	max	.008	3	.256	1	.144	4	0	1	NC	1	NC	1
570			min	-.005	2	-.132	3	0	1	-8.907e-7	4	NC	1	NC	1
571	M9	1	max	.004	3	.111	1	.484	4	1.855e-2	3	NC	1	NC	1
572			min	-.001	10	-.014	3	0	1	-1.808e-2	1	NC	1	NC	1
573		2	max	.004	3	.055	1	.474	4	9.177e-3	3	NC	3	NC	1
574			min	-.001	10	-.007	3	0	12	-8.812e-3	1	2036.645	1	NC	1
575		3	max	.004	3	.005	3	.462	4	1.413e-2	4	NC	5	NC	1
576			min	-.001	10	-.006	1	0	12	-2.005e-5	10	973.648	1	7322.467	4
577		4	max	.004	3	.027	3	.449	4	1.108e-2	5	NC	5	NC	1
578			min	-.001	10	-.077	1	0	12	-5.027e-3	1	607.679	1	5254.076	4
579		5	max	.004	3	.055	3	.436	4	8.312e-3	5	NC	15	NC	1
580			min	-.001	10	-.152	1	0	12	-1.018e-2	1	434.407	1	4192.938	4
581		6	max	.003	3	.085	3	.422	4	9.882e-3	3	NC	15	NC	1
582			min	-.001	10	-.225	1	0	12	-1.534e-2	1	339.645	1	3545.417	4
583		7	max	.003	3	.114	3	.407	4	1.315e-2	3	9545.597	15	NC	1
584			min	-.001	10	-.291	1	0	1	-2.05e-2	1	284.044	1	3104.633	4
585		8	max	.003	3	.139	3	.393	4	1.641e-2	3	8479.716	15	NC	1
586			min	-.001	10	-.344	1	0	1	-2.565e-2	1	251.3	1	2776.477	5
587		9	max	.003	3	.155	3	.377	4	1.649e-2	3	7924.129	15	NC	1
588			min	-.001	10	-.377	1	0	12	-2.818e-2	1	234.308	1	2557.446	4
589		10	max	.003	3	.161	3	.359	4	1.446e-2	3	7754.964	15	NC	1
590			min	-.001	10	-.388	1	0	1	-2.897e-2	1	229.215	1	2512.515	4
591		11	max	.003	3	.157	3	.339	4	1.243e-2	3	7923.982	15	NC	1
592			min	0	10	-.377	1	0	1	-2.976e-2	1	234.564	1	2587.764	4
593		12	max	.003	3	.144	3	.317	4	1.038e-2	3	8479.439	15	NC	1
594			min	0	10	-.343	1	0	12	-2.803e-2	1	252.096	1	2769.971	4
595		13	max	.003	3	.122	3	.292	4	8.307e-3	3	9545.164	15	NC	1
596			min	0	10	-.29	1	0	12	-2.252e-2	1	286.012	1	3292.108	4
597		14	max	.003	3	.095	3	.264	4	6.237e-3	3	NC	15	NC	1
598			min	0	10	-.223	1	-.002	1	-1.702e-2	1	343.886	1	4405.941	5
599		15	max	.003	3	.064	3	.235	4	5.818e-3	5	NC	15	NC	1
600			min	0	10	-.149	1	-.004	1	-1.151e-2	1	443.179	1	6965.446	5
601		16	max	.003	3	.032	3	.206	4	7.849e-3	5	NC	5	NC	1
602			min	0	10	-.074	1	-.006	1	-6.001e-3	1	626.226	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.003	3	.002	3	.181	4	9.952e-3	4	NC	5	NC	1
604		min	0	10	-.004	1	-.007	1	-4.934e-4	1	1015.758	1	NC	1
605	18	max	.003	3	.055	1	.16	4	4.663e-3	5	NC	5	NC	1
606		min	0	10	-.025	3	-.005	1	-1.041e-2	1	2143.851	1	NC	1
607	19	max	.003	3	.108	1	.143	4	6.664e-3	3	NC	1	NC	1
608		min	0	10	-.049	3	0	12	-2.069e-2	1	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1723  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

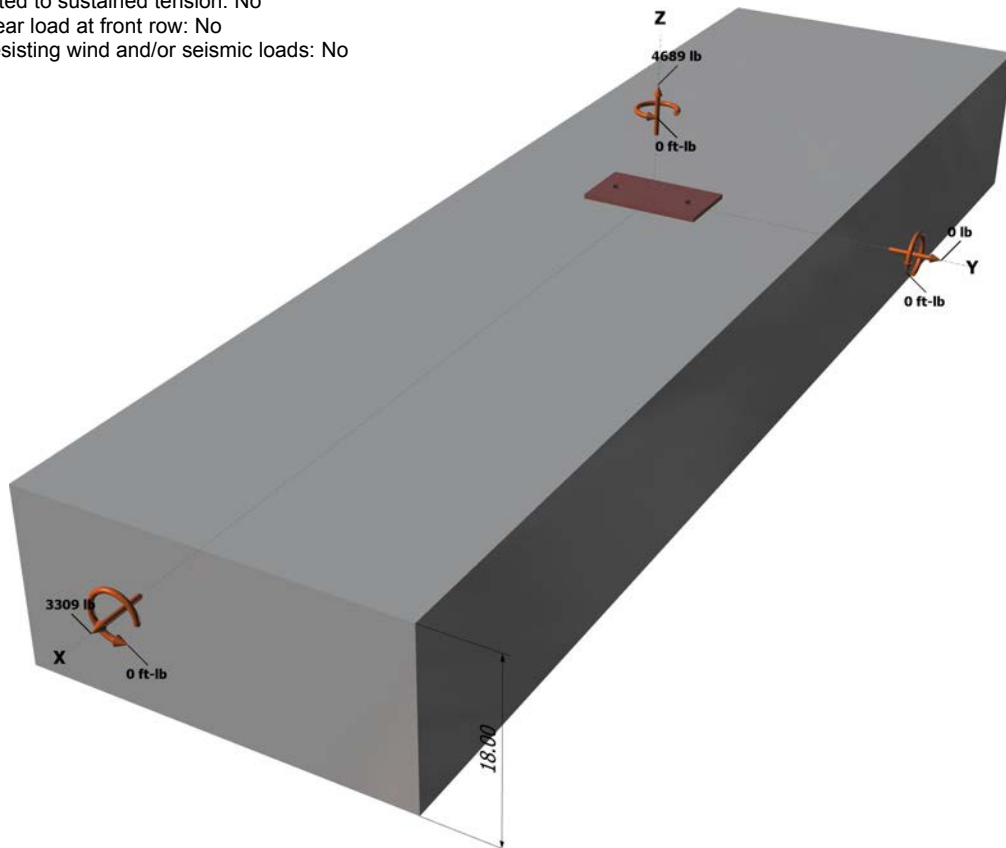
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

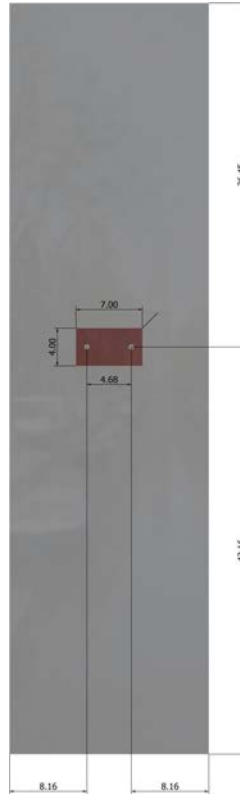
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Anchor Designer™  
Software  
Version 2.4.5673.0

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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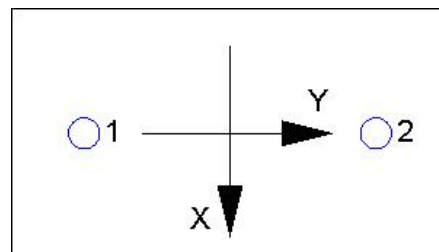
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 4689  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.