



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

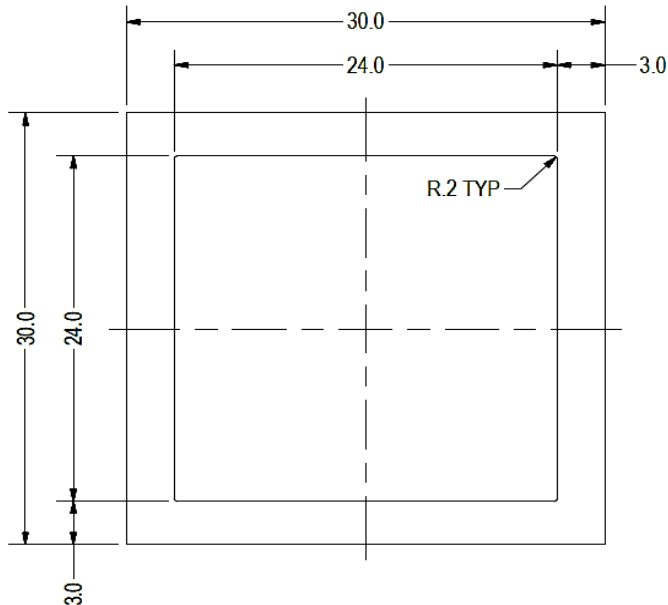
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.001 k-ft
$P_n$ =	0.942 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>8%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.771 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>20%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.806 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	<u>18%</u>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.005 k-ft
$P_n$ =	0.065 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>12%</u>



A cross brace kit is required every 17 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

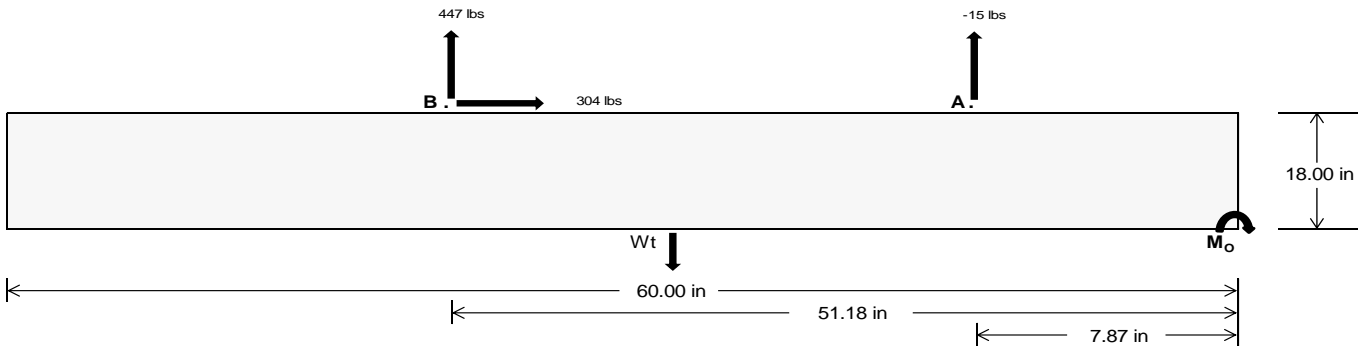
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>31.68</u>	<u>1861.01</u>	k
Compressive Load =	<u>1224.22</u>	<u>1346.85</u>	k
Lateral Load =	<u>4.17</u>	<u>1265.64</u>	k
Moment (Weak Axis) =	<u>0.01</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 28215.9$  in-lbs  
Resisting Force Required = 940.53 lbs  
S.F. = 1.67  
Weight Required = 1567.55 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 304.09 lbs  
Friction = 0.4  
Weight Required = 760.22 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 304.09 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$

Ballast Width			
22 in	23 in	24 in	25 in
1994 lbs	2084 lbs	2175 lbs	2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	480 lbs	480 lbs	480 lbs	480 lbs	362 lbs	362 lbs	362 lbs	362 lbs	583 lbs	583 lbs	583 lbs	583 lbs	30 lbs	30 lbs	30 lbs	30 lbs
$F_B$	323 lbs	323 lbs	323 lbs	323 lbs	584 lbs	584 lbs	584 lbs	584 lbs	647 lbs	647 lbs	647 lbs	647 lbs	-893 lbs	-893 lbs	-893 lbs	-893 lbs
$F_V$	62 lbs	62 lbs	62 lbs	62 lbs	556 lbs	556 lbs	556 lbs	556 lbs	458 lbs	458 lbs	458 lbs	458 lbs	-608 lbs	-608 lbs	-608 lbs	-608 lbs
$P_{total}$	2797 lbs	2888 lbs	2979 lbs	3069 lbs	2939 lbs	3030 lbs	3121 lbs	3211 lbs	3224 lbs	3315 lbs	3405 lbs	3496 lbs	333 lbs	388 lbs	442 lbs	496 lbs
$M$	408 lbs-ft	408 lbs-ft	408 lbs-ft	408 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	471 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft	721 lbs-ft
$e$	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.16 ft	1.86 ft	1.63 ft	1.45 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	251.7 psf	250.2 psf	248.9 psf	247.6 psf	259.0 psf	257.2 psf	255.5 psf	254.0 psf	270.3 psf	268.0 psf	265.9 psf	264.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	358.6 psf	352.5 psf	346.8 psf	341.7 psf	382.4 psf	375.2 psf	368.6 psf	362.6 psf	433.1 psf	423.7 psf	415.1 psf	407.2 psf	359.0 psf	210.4 psf	169.4 psf	151.5 psf

Maximum Bearing Pressure = 433 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

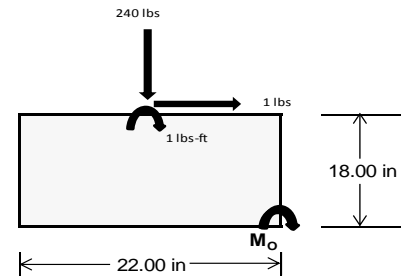
## Overturning Check

$M_o = 217.8 \text{ ft-lbs}$   
 Resisting Force Required = 237.65 lbs  
 S.F. = 1.67  
 Weight Required = 396.09 lbs  
 Minimum Width = 22 in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	81 lbs	202 lbs	77 lbs	245 lbs	673 lbs	240 lbs	24 lbs	59 lbs	22 lbs
$F_v$	4 lbs	4 lbs	0 lbs	15 lbs	14 lbs	1 lbs	1 lbs	1 lbs	0 lbs
$P_{total}$	2549 lbs	2670 lbs	2545 lbs	2594 lbs	3022 lbs	2590 lbs	745 lbs	781 lbs	744 lbs
$M$	6 lbs-ft	6 lbs-ft	0 lbs-ft	25 lbs-ft	21 lbs-ft	2 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.81 ft	1.82 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	276.0 sqft	289.3 sqft	277.5 sqft	274.0 sqft	322.3 sqft	281.7 sqft	80.7 sqft	84.6 sqft	81.1 sqft
$f_{max}$	280.2 psf	293.3 psf	277.8 psf	292.1 psf	337.2 psf	283.4 psf	81.9 psf	85.8 psf	81.2 psf



Maximum Bearing Pressure = 337 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



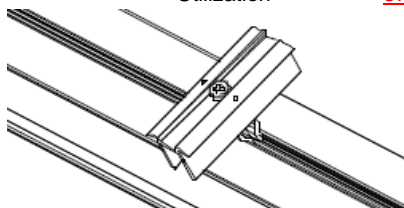
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

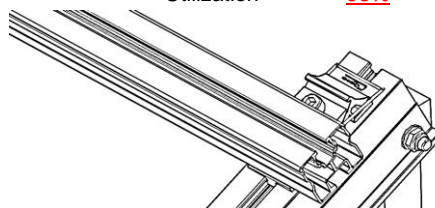
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.454 k
Allowable Uplift =	1.214 k
Utilization =	<u>37%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.088 k
Allowable Uplift =	1.116 k
Utilization =	<u>98%</u>



### 6.2 Bolted Connections

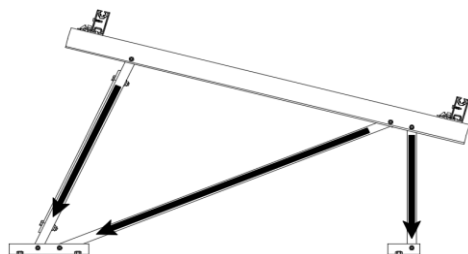
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.942 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

#### Diagonal Strut

Maximum Axial Load =	0.771 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



#### Rear Strut

Maximum Axial Load =	1.164 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.065 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	33.11 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.662 in
Max Drift, $\Delta_{MAX}$ =	0.048 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$210.919$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 81.00 \text{ in}$$

$$J = 0.255$$

$$219.027$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.5$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.6 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.218 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &23.1371 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.13 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.5 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.446 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

# A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	248.065	2	304.487	2	-.003	15	0	15	0	1	0	1
2		min	-304.165	3	-436.131	3	-.129	1	0	3	0	1	0	1
3	N7	max	.026	3	376.613	1	-.076	15	0	15	0	1	0	1
4		min	-.164	2	15.753	15	-1.467	1	-.003	1	0	1	0	1
5	N15	max	.202	3	941.707	1	.651	1	.001	1	0	1	0	1
6		min	-1.628	2	34.716	15	-.497	3	0	3	0	1	0	1
7	N16	max	911.478	2	1036.042	2	-.099	10	0	1	0	1	0	1
8		min	-973.57	3	-1431.543	3	-56.863	3	0	3	0	1	0	1
9	N23	max	.026	3	376.27	1	3.205	1	.006	1	0	1	0	1
10		min	-.164	2	15.901	15	.156	15	0	15	0	1	0	1
11	N24	max	248.481	2	308.496	2	57.262	3	.002	1	0	1	0	1
12		min	-304.355	3	-433.902	3	.01	10	0	3	0	1	0	1
13	Totals:	max	1406.068	2	3091.574	1	0	1						
14		min	-1581.835	3	-2092.668	3	0	2						

### Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	248.764	1	.677	4	.432	1	0	15	0	15	0	1
2			min	-360.262	3	.159	15	-.033	3	0	1	0	1	0	1
3		2	max	248.899	1	.62	4	.432	1	0	15	0	15	0	15
4			min	-360.161	3	.146	15	-.033	3	0	1	0	1	0	4
5		3	max	249.034	1	.562	4	.432	1	0	15	0	15	0	15
6			min	-360.059	3	.132	15	-.033	3	0	1	0	1	0	4
7		4	max	249.169	1	.505	4	.432	1	0	15	0	15	0	15
8			min	-359.958	3	.119	15	-.033	3	0	1	0	1	0	4
9		5	max	249.303	1	.448	4	.432	1	0	15	0	1	0	15
10			min	-359.857	3	.105	15	-.033	3	0	1	0	3	0	4
11		6	max	249.438	1	.39	4	.432	1	0	15	0	1	0	15
12			min	-359.756	3	.092	15	-.033	3	0	1	0	3	0	4
13		7	max	249.573	1	.333	4	.432	1	0	15	0	1	0	15
14			min	-359.655	3	.078	15	-.033	3	0	1	0	3	0	4
15		8	max	249.708	1	.275	4	.432	1	0	15	0	1	0	15
16			min	-359.554	3	.065	15	-.033	3	0	1	0	3	0	4
17		9	max	249.843	1	.218	4	.432	1	0	15	0	1	0	15
18			min	-359.453	3	.051	15	-.033	3	0	1	0	3	0	4
19		10	max	249.978	1	.16	4	.432	1	0	15	0	1	0	15
20			min	-359.351	3	.038	15	-.033	3	0	1	0	3	0	4
21		11	max	250.113	1	.108	2	.432	1	0	15	0	1	0	15
22			min	-359.25	3	.016	12	-.033	3	0	1	0	3	0	4
23		12	max	250.247	1	.063	2	.432	1	0	15	0	1	0	15
24			min	-359.149	3	-.013	3	-.033	3	0	1	0	3	0	4
25		13	max	250.382	1	.018	2	.432	1	0	15	0	1	0	15
26			min	-359.048	3	-.046	3	-.033	3	0	1	0	3	0	4
27		14	max	250.517	1	-.016	15	.432	1	0	15	0	1	0	15
28			min	-358.947	3	-.08	3	-.033	3	0	1	0	3	0	4
29		15	max	250.652	1	-.03	15	.432	1	0	15	0	1	0	15
30			min	-358.846	3	-.127	4	-.033	3	0	1	0	3	0	4
31		16	max	250.787	1	-.043	15	.432	1	0	15	0	1	0	15
32			min	-358.745	3	-.185	4	-.033	3	0	1	0	3	0	4
33		17	max	250.922	1	-.057	15	.432	1	0	15	.001	1	0	15
34			min	-358.643	3	-.242	4	-.033	3	0	1	0	3	0	4
35		18	max	251.057	1	-.07	15	.432	1	0	15	.001	1	0	15
36			min	-358.542	3	-.3	4	-.033	3	0	1	0	3	0	4
37		19	max	251.192	1	-.084	15	.432	1	0	15	.001	1	0	15



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-358.441	3	-.357	4	-.033	3	0	1	0	3	0	4
39	M3	1	max	200.341	2	1.735	4	-.022	15	0	15	.002	1	0
40		min	-213.674	3	.408	15	-.484	1	0	1	0	15	0	15
41		2	max	200.271	2	1.559	4	-.022	15	0	15	.002	1	0
42		min	-213.726	3	.367	15	-.484	1	0	1	0	15	0	3
43		3	max	200.201	2	1.382	4	-.022	15	0	15	.001	1	0
44		min	-213.779	3	.325	15	-.484	1	0	1	0	15	0	3
45		4	max	200.131	2	1.206	4	-.022	15	0	15	.001	1	0
46		min	-213.831	3	.284	15	-.484	1	0	1	0	15	0	4
47		5	max	200.061	2	1.029	4	-.022	15	0	15	.001	1	0
48		min	-213.884	3	.242	15	-.484	1	0	1	0	15	0	4
49		6	max	199.991	2	.853	4	-.022	15	0	15	.001	1	0
50		min	-213.936	3	.201	15	-.484	1	0	1	0	15	0	4
51		7	max	199.921	2	.677	4	-.022	15	0	15	.001	1	0
52		min	-213.989	3	.159	15	-.484	1	0	1	0	15	0	4
53		8	max	199.851	2	.5	4	-.022	15	0	15	0	1	0
54		min	-214.041	3	.118	15	-.484	1	0	1	0	15	-.001	4
55		9	max	199.781	2	.324	4	-.022	15	0	15	0	1	0
56		min	-214.094	3	.076	15	-.484	1	0	1	0	15	-.001	4
57		10	max	199.711	2	.148	4	-.022	15	0	15	0	1	0
58		min	-214.146	3	.035	15	-.484	1	0	1	0	15	-.001	4
59		11	max	199.641	2	.003	2	-.022	15	0	15	0	1	0
60		min	-214.199	3	-.053	3	-.484	1	0	1	0	15	-.001	4
61		12	max	199.571	2	-.048	15	-.022	15	0	15	0	1	0
62		min	-214.251	3	-.205	4	-.484	1	0	1	0	15	-.001	4
63		13	max	199.501	2	-.09	15	-.022	15	0	15	0	1	0
64		min	-214.304	3	-.382	4	-.484	1	0	1	0	15	-.001	4
65		14	max	199.431	2	-.131	15	-.022	15	0	15	0	1	0
66		min	-214.356	3	-.558	4	-.484	1	0	1	0	15	-.001	4
67		15	max	199.361	2	-.172	15	-.022	15	0	15	0	1	0
68		min	-214.409	3	-.734	4	-.484	1	0	1	0	15	0	4
69		16	max	199.291	2	-.214	15	-.022	15	0	15	0	1	0
70		min	-214.461	3	-.911	4	-.484	1	0	1	0	12	0	4
71		17	max	199.221	2	-.255	15	-.022	15	0	15	0	15	0
72		min	-214.514	3	-1.087	4	-.484	1	0	1	0	1	0	4
73		18	max	199.151	2	-.297	15	-.022	15	0	15	0	15	0
74		min	-214.566	3	-1.263	4	-.484	1	0	1	0	1	0	4
75		19	max	199.081	2	-.338	15	-.022	15	0	15	0	15	0
76		min	-214.619	3	-1.44	4	-.484	1	0	1	0	1	0	1
77	M4	1	max	375.449	1	0	1	-.076	15	0	1	0	3	0
78		min	15.402	15	0	1	-1.566	1	0	1	0	2	0	1
79		2	max	375.513	1	0	1	-.076	15	0	1	0	15	0
80		min	15.421	15	0	1	-1.566	1	0	1	0	1	0	1
81		3	max	375.578	1	0	1	-.076	15	0	1	0	15	0
82		min	15.441	15	0	1	-1.566	1	0	1	0	1	0	1
83		4	max	375.643	1	0	1	-.076	15	0	1	0	15	0
84		min	15.46	15	0	1	-1.566	1	0	1	0	1	0	1
85		5	max	375.707	1	0	1	-.076	15	0	1	0	15	0
86		min	15.48	15	0	1	-1.566	1	0	1	0	1	0	1
87		6	max	375.772	1	0	1	-.076	15	0	1	0	15	0
88		min	15.499	15	0	1	-1.566	1	0	1	0	1	0	1
89		7	max	375.837	1	0	1	-.076	15	0	1	0	15	0
90		min	15.519	15	0	1	-1.566	1	0	1	0	1	0	1
91		8	max	375.902	1	0	1	-.076	15	0	1	0	15	0
92		min	15.538	15	0	1	-1.566	1	0	1	-.001	1	0	1
93		9	max	375.966	1	0	1	-.076	15	0	1	0	15	0
94		min	15.558	15	0	1	-1.566	1	0	1	-.001	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95		10	max	376.031	1	0	1	-.076	15	0	1	0	15	0	1
96			min	15.577	15	0	1	-1.566	1	0	1	-.001	1	0	1
97		11	max	376.096	1	0	1	-.076	15	0	1	0	15	0	1
98			min	15.597	15	0	1	-1.566	1	0	1	-.001	1	0	1
99		12	max	376.16	1	0	1	-.076	15	0	1	0	15	0	1
100			min	15.616	15	0	1	-1.566	1	0	1	-.002	1	0	1
101		13	max	376.225	1	0	1	-.076	15	0	1	0	15	0	1
102			min	15.636	15	0	1	-1.566	1	0	1	-.002	1	0	1
103		14	max	376.29	1	0	1	-.076	15	0	1	0	15	0	1
104			min	15.655	15	0	1	-1.566	1	0	1	-.002	1	0	1
105		15	max	376.354	1	0	1	-.076	15	0	1	0	15	0	1
106			min	15.675	15	0	1	-1.566	1	0	1	-.002	1	0	1
107		16	max	376.419	1	0	1	-.076	15	0	1	0	15	0	1
108			min	15.695	15	0	1	-1.566	1	0	1	-.002	1	0	1
109		17	max	376.484	1	0	1	-.076	15	0	1	0	15	0	1
110			min	15.714	15	0	1	-1.566	1	0	1	-.002	1	0	1
111		18	max	376.549	1	0	1	-.076	15	0	1	0	15	0	1
112			min	15.734	15	0	1	-1.566	1	0	1	-.002	1	0	1
113		19	max	376.613	1	0	1	-.076	15	0	1	0	15	0	1
114			min	15.753	15	0	1	-1.566	1	0	1	-.003	1	0	1
115	M6	1	max	803.773	1	.682	4	.137	1	0	3	0	3	0	1
116			min	-1163.955	3	.16	15	-.159	3	0	15	0	11	0	1
117		2	max	803.908	1	.624	4	.137	1	0	3	0	3	0	15
118			min	-1163.853	3	.146	15	-.159	3	0	15	0	11	0	4
119		3	max	804.042	1	.567	4	.137	1	0	3	0	3	0	15
120			min	-1163.752	3	.133	15	-.159	3	0	15	0	11	0	4
121		4	max	804.177	1	.509	4	.137	1	0	3	0	3	0	15
122			min	-1163.651	3	.119	15	-.159	3	0	15	0	15	0	4
123		5	max	804.312	1	.452	4	.137	1	0	3	0	3	0	15
124			min	-1163.55	3	.097	12	-.159	3	0	15	0	10	0	4
125		6	max	804.447	1	.401	2	.137	1	0	3	0	1	0	15
126			min	-1163.449	3	.075	12	-.159	3	0	15	0	10	0	4
127		7	max	804.582	1	.357	2	.137	1	0	3	0	1	0	15
128			min	-1163.348	3	.052	12	-.159	3	0	15	0	10	0	4
129		8	max	804.717	1	.312	2	.137	1	0	3	0	1	0	12
130			min	-1163.247	3	.03	12	-.159	3	0	15	0	3	0	4
131		9	max	804.852	1	.267	2	.137	1	0	3	0	1	0	12
132			min	-1163.145	3	.004	3	-.159	3	0	15	0	3	0	4
133		10	max	804.986	1	.222	2	.137	1	0	3	0	1	0	12
134			min	-1163.044	3	-.03	3	-.159	3	0	15	0	3	0	2
135		11	max	805.121	1	.177	2	.137	1	0	3	0	1	0	12
136			min	-1162.943	3	-.063	3	-.159	3	0	15	0	3	0	2
137		12	max	805.256	1	.133	2	.137	1	0	3	0	1	0	12
138			min	-1162.842	3	-.097	3	-.159	3	0	15	0	3	0	2
139		13	max	805.391	1	.088	2	.137	1	0	3	0	1	0	12
140			min	-1162.741	3	-.131	3	-.159	3	0	15	0	3	0	2
141		14	max	805.526	1	.043	2	.137	1	0	3	0	1	0	12
142			min	-1162.64	3	-.164	3	-.159	3	0	15	0	3	0	2
143		15	max	805.661	1	-.002	2	.137	1	0	3	0	1	0	12
144			min	-1162.539	3	-.198	3	-.159	3	0	15	0	3	0	2
145		16	max	805.796	1	-.043	15	.137	1	0	3	0	1	0	12
146			min	-1162.437	3	-.231	3	-.159	3	0	15	0	3	0	2
147		17	max	805.931	1	-.056	15	.137	1	0	3	0	1	0	3
148			min	-1162.336	3	-.265	3	-.159	3	0	15	0	3	0	2
149		18	max	806.065	1	-.07	15	.137	1	0	3	0	1	0	3
150			min	-1162.235	3	-.299	3	-.159	3	0	15	0	3	0	2
151		19	max	806.2	1	-.083	15	.137	1	0	3	0	1	0	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1162.134	3	-.353	4	-.159	3	0	15	0	3	0	2
153	M7	1	max	770.5	2	1.74	4	.03	3	0	1	0	2	2
154		min	-668.996	3	.409	15	-.008	2	0	3	0	3	0	3
155		2	max	770.43	2	1.564	4	.03	3	0	1	0	2	2
156		min	-669.048	3	.367	15	-.008	2	0	3	0	3	0	3
157		3	max	770.36	2	1.387	4	.03	3	0	1	0	2	2
158		min	-669.101	3	.326	15	-.008	2	0	3	0	3	0	3
159		4	max	770.29	2	1.211	4	.03	3	0	1	0	2	2
160		min	-669.153	3	.284	15	-.008	2	0	3	0	3	0	3
161		5	max	770.22	2	1.035	4	.03	3	0	1	0	2	15
162		min	-669.206	3	.243	15	-.008	2	0	3	0	3	0	3
163		6	max	770.15	2	.858	4	.03	3	0	1	0	2	15
164		min	-669.258	3	.202	15	-.008	2	0	3	0	3	0	3
165		7	max	770.08	2	.682	4	.03	3	0	1	0	2	15
166		min	-669.311	3	.16	15	-.008	2	0	3	0	3	0	4
167		8	max	770.01	2	.506	4	.03	3	0	1	0	2	15
168		min	-669.363	3	.119	15	-.008	2	0	3	0	3	-.001	4
169		9	max	769.94	2	.348	2	.03	3	0	1	0	2	15
170		min	-669.416	3	.052	12	-.008	2	0	3	0	3	-.001	4
171		10	max	769.87	2	.21	2	.03	3	0	1	0	2	15
172		min	-669.468	3	-.032	3	-.008	2	0	3	0	3	-.001	4
173		11	max	769.8	2	.073	2	.03	3	0	1	0	2	15
174		min	-669.521	3	-.136	3	-.008	2	0	3	0	3	-.001	4
175		12	max	769.73	2	-.047	15	.03	3	0	1	0	2	15
176		min	-669.573	3	-.239	3	-.008	2	0	3	0	3	-.001	4
177		13	max	769.66	2	-.089	15	.03	3	0	1	0	2	15
178		min	-669.626	3	-.376	4	-.008	2	0	3	0	3	-.001	4
179		14	max	769.59	2	-.13	15	.03	3	0	1	0	11	15
180		min	-669.678	3	-.553	4	-.008	2	0	3	0	3	-.001	4
181		15	max	769.52	2	-.172	15	.03	3	0	1	0	11	15
182		min	-669.731	3	-.729	4	-.008	2	0	3	0	3	0	4
183		16	max	769.45	2	-.213	15	.03	3	0	1	0	11	15
184		min	-669.783	3	-.905	4	-.008	2	0	3	0	3	0	4
185		17	max	769.38	2	-.255	15	.03	3	0	1	0	11	15
186		min	-669.836	3	-1.082	4	-.008	2	0	3	0	3	0	4
187		18	max	769.31	2	-.296	15	.03	3	0	1	0	11	15
188		min	-669.888	3	-1.258	4	-.008	2	0	3	0	3	0	4
189		19	max	769.24	2	-.337	15	.03	3	0	1	0	11	1
190		min	-669.941	3	-1.435	4	-.008	2	0	3	0	3	0	1
191	M8	1	max	940.542	1	0	1	.769	1	0	1	0	15	1
192		min	34.365	15	0	1	-.507	3	0	1	0	1	0	1
193		2	max	940.607	1	0	1	.769	1	0	1	0	1	1
194		min	34.384	15	0	1	-.507	3	0	1	0	3	0	1
195		3	max	940.672	1	0	1	.769	1	0	1	0	1	1
196		min	34.404	15	0	1	-.507	3	0	1	0	3	0	1
197		4	max	940.736	1	0	1	.769	1	0	1	0	1	1
198		min	34.423	15	0	1	-.507	3	0	1	0	3	0	1
199		5	max	940.801	1	0	1	.769	1	0	1	0	1	1
200		min	34.443	15	0	1	-.507	3	0	1	0	3	0	1
201		6	max	940.866	1	0	1	.769	1	0	1	0	1	1
202		min	34.462	15	0	1	-.507	3	0	1	0	3	0	1
203		7	max	940.931	1	0	1	.769	1	0	1	0	1	1
204		min	34.482	15	0	1	-.507	3	0	1	0	3	0	1
205		8	max	940.995	1	0	1	.769	1	0	1	0	1	1
206		min	34.501	15	0	1	-.507	3	0	1	0	3	0	1
207		9	max	941.06	1	0	1	.769	1	0	1	0	1	1
208		min	34.521	15	0	1	-.507	3	0	1	0	3	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	941.125	1	0	1	.769	1	0	1	0	1	0	1
210		min	34.54	15	0	1	-.507	3	0	1	0	3	0	1
211	11	max	941.189	1	0	1	.769	1	0	1	0	1	0	1
212		min	34.56	15	0	1	-.507	3	0	1	0	3	0	1
213	12	max	941.254	1	0	1	.769	1	0	1	0	1	0	1
214		min	34.579	15	0	1	-.507	3	0	1	0	3	0	1
215	13	max	941.319	1	0	1	.769	1	0	1	0	1	0	1
216		min	34.599	15	0	1	-.507	3	0	1	0	3	0	1
217	14	max	941.383	1	0	1	.769	1	0	1	0	1	0	1
218		min	34.618	15	0	1	-.507	3	0	1	0	3	0	1
219	15	max	941.448	1	0	1	.769	1	0	1	0	1	0	1
220		min	34.638	15	0	1	-.507	3	0	1	0	3	0	1
221	16	max	941.513	1	0	1	.769	1	0	1	.001	1	0	1
222		min	34.657	15	0	1	-.507	3	0	1	0	3	0	1
223	17	max	941.578	1	0	1	.769	1	0	1	.001	1	0	1
224		min	34.677	15	0	1	-.507	3	0	1	0	3	0	1
225	18	max	941.642	1	0	1	.769	1	0	1	.001	1	0	1
226		min	34.697	15	0	1	-.507	3	0	1	0	3	0	1
227	19	max	941.707	1	0	1	.769	1	0	1	.001	1	0	1
228		min	34.716	15	0	1	-.507	3	0	1	0	3	0	1
229	M10	1	max	258.397	1	.673	.006	3	0	1	0	1	0	1
230		min	-329.194	3	.159	15	-.184	1	0	3	0	3	0	1
231	2	max	258.531	1	.616	4	.006	3	0	1	0	1	0	15
232		min	-329.093	3	.145	15	-.184	1	0	3	0	3	0	4
233	3	max	258.666	1	.558	4	.006	3	0	1	0	1	0	15
234		min	-328.992	3	.132	15	-.184	1	0	3	0	3	0	4
235	4	max	258.801	1	.501	4	.006	3	0	1	0	1	0	15
236		min	-328.891	3	.118	15	-.184	1	0	3	0	3	0	4
237	5	max	258.936	1	.443	4	.006	3	0	1	0	1	0	15
238		min	-328.789	3	.105	15	-.184	1	0	3	0	3	0	4
239	6	max	259.071	1	.386	4	.006	3	0	1	0	1	0	15
240		min	-328.688	3	.091	15	-.184	1	0	3	0	3	0	4
241	7	max	259.206	1	.328	4	.006	3	0	1	0	1	0	15
242		min	-328.587	3	.078	15	-.184	1	0	3	0	3	0	4
243	8	max	259.341	1	.271	4	.006	3	0	1	0	1	0	15
244		min	-328.486	3	.064	15	-.184	1	0	3	0	3	0	4
245	9	max	259.475	1	.213	4	.006	3	0	1	0	1	0	15
246		min	-328.385	3	.051	15	-.184	1	0	3	0	3	0	4
247	10	max	259.61	1	.156	4	.006	3	0	1	0	1	0	15
248		min	-328.284	3	.037	15	-.184	1	0	3	0	3	0	4
249	11	max	259.745	1	.108	2	.006	3	0	1	0	11	0	15
250		min	-328.183	3	.024	15	-.184	1	0	3	0	3	0	4
251	12	max	259.88	1	.063	2	.006	3	0	1	0	11	0	15
252		min	-328.081	3	.007	12	-.184	1	0	3	0	3	0	4
253	13	max	260.015	1	.018	2	.006	3	0	1	0	15	0	15
254		min	-327.98	3	-.023	3	-.184	1	0	3	0	3	0	4
255	14	max	260.15	1	-.017	15	.006	3	0	1	0	15	0	15
256		min	-327.879	3	-.074	4	-.184	1	0	3	0	3	0	4
257	15	max	260.285	1	-.03	15	.006	3	0	1	0	15	0	15
258		min	-327.778	3	-.132	4	-.184	1	0	3	0	3	0	4
259	16	max	260.42	1	-.044	15	.006	3	0	1	0	15	0	15
260		min	-327.677	3	-.189	4	-.184	1	0	3	0	1	0	4
261	17	max	260.554	1	-.058	15	.006	3	0	1	0	15	0	15
262		min	-327.576	3	-.247	4	-.184	1	0	3	0	1	0	4
263	18	max	260.689	1	-.071	15	.006	3	0	1	0	15	0	15
264		min	-327.474	3	-.304	4	-.184	1	0	3	0	1	0	4
265	19	max	260.824	1	-.085	15	.006	3	0	1	0	15	0	15



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-327.373	3	-.361	4	-.184	1	0	3	0	1	0	4
267	M11	1	max	200.029	2	1.739	4	.549	1	0	1	0	3	0	4
268			min	-214.33	3	.409	15	-.019	3	0	15	-.002	1	0	12
269		2	max	199.959	2	1.562	4	.549	1	0	1	0	3	0	1
270			min	-214.383	3	.367	15	-.019	3	0	15	-.002	1	0	3
271		3	max	199.889	2	1.386	4	.549	1	0	1	0	3	0	1
272			min	-214.435	3	.326	15	-.019	3	0	15	-.001	1	0	3
273		4	max	199.819	2	1.21	4	.549	1	0	1	0	3	0	15
274			min	-214.488	3	.284	15	-.019	3	0	15	-.001	1	0	3
275		5	max	199.749	2	1.033	4	.549	1	0	1	0	3	0	15
276			min	-214.54	3	.243	15	-.019	3	0	15	-.001	1	0	4
277		6	max	199.679	2	.857	4	.549	1	0	1	0	3	0	15
278			min	-214.593	3	.201	15	-.019	3	0	15	-.001	1	0	4
279		7	max	199.609	2	.681	4	.549	1	0	1	0	3	0	15
280			min	-214.645	3	.16	15	-.019	3	0	15	-.001	1	0	4
281		8	max	199.539	2	.504	4	.549	1	0	1	0	3	0	15
282			min	-214.698	3	.118	15	-.019	3	0	15	0	1	-.001	4
283		9	max	199.469	2	.328	4	.549	1	0	1	0	3	0	15
284			min	-214.75	3	.077	15	-.019	3	0	15	0	1	-.001	4
285		10	max	199.399	2	.151	4	.549	1	0	1	0	3	0	15
286			min	-214.803	3	.024	12	-.019	3	0	15	0	1	-.001	4
287		11	max	199.329	2	.005	1	.549	1	0	1	0	3	0	15
288			min	-214.855	3	-.07	3	-.019	3	0	15	0	1	-.001	4
289		12	max	199.259	2	-.047	15	.549	1	0	1	0	3	0	15
290			min	-214.908	3	-.201	4	-.019	3	0	15	0	1	-.001	4
291		13	max	199.189	2	-.089	15	.549	1	0	1	0	3	0	15
292			min	-214.96	3	-.378	4	-.019	3	0	15	0	1	-.001	4
293		14	max	199.119	2	-.13	15	.549	1	0	1	0	3	0	15
294			min	-215.013	3	-.554	4	-.019	3	0	15	0	1	-.001	4
295		15	max	199.049	2	-.172	15	.549	1	0	1	0	3	0	15
296			min	-215.065	3	-.73	4	-.019	3	0	15	0	1	0	4
297		16	max	198.979	2	-.213	15	.549	1	0	1	0	3	0	15
298			min	-215.118	3	-.907	4	-.019	3	0	15	0	10	0	4
299		17	max	198.909	2	-.255	15	.549	1	0	1	0	3	0	15
300			min	-215.17	3	-1.083	4	-.019	3	0	15	0	15	0	4
301		18	max	198.839	2	-.296	15	.549	1	0	1	0	1	0	15
302			min	-215.223	3	-1.26	4	-.019	3	0	15	0	15	0	4
303		19	max	198.769	2	-.338	15	.549	1	0	1	0	1	0	1
304			min	-215.275	3	-1.436	4	-.019	3	0	15	0	15	0	1
305	M12	1	max	375.105	1	0	1	3.417	1	0	1	0	2	0	1
306			min	15.55	15	0	1	.156	15	0	1	0	3	0	1
307		2	max	375.17	1	0	1	3.417	1	0	1	0	1	0	1
308			min	15.569	15	0	1	.156	15	0	1	0	15	0	1
309		3	max	375.235	1	0	1	3.417	1	0	1	0	1	0	1
310			min	15.589	15	0	1	.156	15	0	1	0	15	0	1
311		4	max	375.3	1	0	1	3.417	1	0	1	0	1	0	1
312			min	15.608	15	0	1	.156	15	0	1	0	15	0	1
313		5	max	375.364	1	0	1	3.417	1	0	1	.001	1	0	1
314			min	15.628	15	0	1	.156	15	0	1	0	15	0	1
315		6	max	375.429	1	0	1	3.417	1	0	1	.002	1	0	1
316			min	15.647	15	0	1	.156	15	0	1	0	15	0	1
317		7	max	375.494	1	0	1	3.417	1	0	1	.002	1	0	1
318			min	15.667	15	0	1	.156	15	0	1	0	15	0	1
319		8	max	375.558	1	0	1	3.417	1	0	1	.002	1	0	1
320			min	15.686	15	0	1	.156	15	0	1	0	15	0	1
321		9	max	375.623	1	0	1	3.417	1	0	1	.002	1	0	1
322			min	15.706	15	0	1	.156	15	0	1	0	15	0	1











Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	3.119	15	-245.006	1	6.334	15	0	2	.006	15	0	3
495		max	-1.046	12	355.168	2	-6.295	15	0	3	.131	1	0	2
496		min	-68.682	1	-163.821	3	-138.871	1	0	2	.006	15	0	3
497		2 max	-1.046	12	250.724	2	-4.825	15	0	3	.039	1	.105	3
498		min	-68.682	1	-115.806	3	-106.292	1	0	2	.002	15	-.227	2
499		3 max	-1.046	12	146.279	2	-3.354	15	0	3	0	12	.174	3
500		min	-68.682	1	-67.792	3	-73.714	1	0	2	-.028	1	-.376	2
501		4 max	-1.046	12	41.835	2	-1.884	15	0	3	-.003	15	.207	3
502		min	-68.682	1	-19.777	3	-41.136	1	0	2	-.071	1	-.447	2
503		5 max	-1.046	12	28.237	3	-.411	10	0	3	-.004	15	.203	3
504		min	-68.682	1	-62.61	2	-8.558	1	0	2	-.09	1	-.439	2
505		6 max	-1.046	12	76.251	3	24.02	1	0	3	-.004	15	.164	3
506		min	-68.682	1	-167.054	2	.333	12	0	2	-.084	1	-.353	2
507		7 max	-1.046	12	124.266	3	56.598	1	0	3	-.002	15	.089	3
508		min	-68.682	1	-271.498	2	1.76	12	0	2	-.054	1	-.188	2
509		8 max	-1.046	12	172.28	3	89.176	1	0	3	.002	2	.055	2
510		min	-68.682	1	-375.943	2	3.186	12	0	2	-.004	3	-.022	3
511		9 max	-1.046	12	220.295	3	121.754	1	0	3	.08	1	.376	2
512		min	-68.682	1	-480.387	2	4.613	12	0	2	0	12	-.169	3
513		10 max	-3.198	15	-11.766	15	154.332	1	0	15	.184	1	.775	2
514		min	-69.622	1	-584.831	2	-9.49	3	0	2	.007	12	-.353	3
515		11 max	-3.198	15	480.387	2	-4.924	12	0	2	.08	1	.376	2
516		min	-69.622	1	-220.295	3	-121.356	1	0	3	.003	12	-.169	3
517		12 max	-3.198	15	375.943	2	-3.497	12	0	2	.002	2	.055	2
518		min	-69.622	1	-172.28	3	-88.778	1	0	3	0	3	-.022	3
519		13 max	-3.198	15	271.498	2	-2.07	12	0	2	-.002	15	.089	3
520		min	-69.622	1	-124.266	3	-56.2	1	0	3	-.054	1	-.188	2
521		14 max	-3.198	15	167.054	2	-.644	12	0	2	-.003	12	.164	3
522		min	-69.622	1	-76.251	3	-23.622	1	0	3	-.084	1	-.353	2
523		15 max	-3.198	15	62.609	2	8.956	1	0	2	-.003	12	.203	3
524		min	-69.622	1	-28.237	3	.428	15	0	3	-.089	1	-.439	2
525		16 max	-3.198	15	19.778	3	41.534	1	0	2	-.002	12	.207	3
526		min	-69.622	1	-41.835	2	1.899	15	0	3	-.07	1	-.447	2
527		17 max	-3.198	15	67.792	3	74.112	1	0	2	0	3	.174	3
528		min	-69.622	1	-146.279	2	3.369	15	0	3	-.027	1	-.376	2
529		18 max	-3.198	15	115.806	3	106.69	1	0	2	.041	1	.105	3
530		min	-69.622	1	-250.724	2	4.84	15	0	3	.002	15	-.227	2
531		19 max	-3.198	15	163.821	3	139.268	1	0	2	.133	1	0	2
532		min	-69.622	1	-355.168	2	6.31	15	0	3	.006	15	0	3
533	M15	1 max	0	2	2.371	4	.049	3	0	1	0	1	0	1
534		min	-63.965	3	0	2	-.039	1	0	3	0	3	0	1
535		2 max	0	2	2.107	4	.049	3	0	1	0	1	0	2
536		min	-64.04	3	0	2	-.039	1	0	3	0	3	0	4
537		3 max	0	2	1.844	4	.049	3	0	1	0	1	0	2
538		min	-64.116	3	0	2	-.039	1	0	3	0	3	-.002	4
539		4 max	0	2	1.58	4	.049	3	0	1	0	1	0	2
540		min	-64.191	3	0	2	-.039	1	0	3	0	3	-.003	4
541		5 max	0	2	1.317	4	.049	3	0	1	0	1	0	2
542		min	-64.267	3	0	2	-.039	1	0	3	0	3	-.003	4
543		6 max	0	2	1.054	4	.049	3	0	1	0	1	0	2
544		min	-64.343	3	0	2	-.039	1	0	3	0	3	-.004	4
545		7 max	0	2	.79	4	.049	3	0	1	0	3	0	2
546		min	-64.418	3	0	2	-.039	1	0	3	0	1	-.004	4
547		8 max	0	2	.527	4	.049	3	0	1	0	3	0	2
548		min	-64.494	3	0	2	-.039	1	0	3	0	1	-.004	4
549		9 max	0	2	.263	4	.049	3	0	1	0	3	0	2
550		min	-64.569	3	0	2	-.039	1	0	3	0	1	-.004	4







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-63.188	3	-2.371	4	-0.02	3	0	2	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.003	1	.01	2	.013	1	-5.169e-5	15	NC	3	NC	3
2			min	-.004	3	-.011	3	-.001	3	-1.12e-3	1	4083.109	2	3196.295	1
3		2	max	.002	1	.009	2	.012	1	-4.94e-5	15	NC	3	NC	3
4			min	-.004	3	-.01	3	-.001	3	-1.071e-3	1	4470.284	2	3431.114	1
5		3	max	.002	1	.009	2	.011	1	-4.711e-5	15	NC	1	NC	3
6			min	-.003	3	-.01	3	-.001	3	-1.022e-3	1	4933.423	2	3709.302	1
7		4	max	.002	1	.008	2	.011	1	-4.481e-5	15	NC	1	NC	2
8			min	-.003	3	-.01	3	0	3	-9.724e-4	1	5491.327	2	4041.046	1
9		5	max	.002	1	.007	2	.01	1	-4.252e-5	15	NC	1	NC	2
10			min	-.003	3	-.009	3	0	3	-9.231e-4	1	6169.194	2	4439.951	1
11		6	max	.002	1	.006	2	.009	1	-4.022e-5	15	NC	1	NC	2
12			min	-.003	3	-.009	3	0	3	-8.738e-4	1	7001.351	2	4924.502	1
13		7	max	.002	1	.005	2	.008	1	-3.793e-5	15	NC	1	NC	2
14			min	-.003	3	-.008	3	0	3	-8.245e-4	1	8035.459	2	5520.326	1
15		8	max	.002	1	.005	2	.007	1	-3.564e-5	15	NC	1	NC	2
16			min	-.002	3	-.008	3	0	3	-7.752e-4	1	9339.174	2	6263.785	1
17		9	max	.001	1	.004	2	.006	1	-3.334e-5	15	NC	1	NC	2
18			min	-.002	3	-.007	3	0	3	-7.259e-4	1	NC	1	7207.921	1
19		10	max	.001	1	.003	2	.005	1	-3.105e-5	15	NC	1	NC	2
20			min	-.002	3	-.007	3	0	3	-6.766e-4	1	NC	1	8432.664	1
21		11	max	.001	1	.003	2	.004	1	-2.875e-5	15	NC	1	NC	1
22			min	-.002	3	-.006	3	0	3	-6.273e-4	1	NC	1	NC	1
23		12	max	.001	1	.002	2	.003	1	-2.646e-5	15	NC	1	NC	1
24			min	-.001	3	-.005	3	0	3	-5.78e-4	1	NC	1	NC	1
25		13	max	0	1	.002	2	.003	1	-2.417e-5	15	NC	1	NC	1
26			min	-.001	3	-.005	3	0	3	-5.287e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	.002	1	-2.187e-5	15	NC	1	NC	1
28			min	-.001	3	-.004	3	0	3	-4.794e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	.001	1	-1.958e-5	15	NC	1	NC	1
30			min	0	3	-.003	3	0	3	-4.301e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	0	1	-1.728e-5	15	NC	1	NC	1
32			min	0	3	-.002	3	0	3	-3.808e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	-1.499e-5	15	NC	1	NC	1
34			min	0	3	-.002	3	0	3	-3.316e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	-1.27e-5	15	NC	1	NC	1
36			min	0	3	0	3	0	12	-2.823e-4	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	-8.371e-6	12	NC	1	NC	1
38			min	0	1	0	1	0	1	-2.33e-4	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	1.116e-4	1	NC	1	NC	1
40			min	0	1	0	1	0	1	4.138e-6	12	NC	1	NC	1
41		2	max	0	3	0	2	0	12	1.349e-4	1	NC	1	NC	1
42			min	0	2	0	3	0	1	6.069e-6	15	NC	1	NC	1
43		3	max	0	3	0	2	0	12	1.582e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	7.155e-6	15	NC	1	NC	1
45		4	max	0	3	0	2	0	12	1.815e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	0	1	8.241e-6	15	NC	1	NC	1
47		5	max	0	3	0	2	0	3	2.048e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	9.327e-6	15	NC	1	NC	1
49		6	max	0	3	0	2	0	3	2.282e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	1.041e-5	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	2.515e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	1	1.15e-5	15	NC	1	NC	1
53		8	max	0	3	0	2	0	3	2.748e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	1.258e-5	15	NC	1	NC	1
55		9	max	.001	3	.001	2	0	3	2.981e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	1	1.367e-5	15	NC	1	NC	1
57		10	max	.001	3	.002	2	0	1	3.214e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	15	1.476e-5	15	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	3.447e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	15	1.584e-5	15	NC	1	NC	1
61		12	max	.001	3	.003	2	.002	1	3.681e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	15	1.693e-5	15	NC	1	NC	1
63		13	max	.002	3	.004	2	.002	1	3.914e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	15	1.801e-5	15	NC	1	NC	1
65		14	max	.002	3	.004	2	.003	1	4.147e-4	1	NC	1	NC	1
66			min	-.002	2	-.008	3	0	15	1.91e-5	15	NC	1	NC	1
67		15	max	.002	3	.005	2	.004	1	4.38e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	15	2.018e-5	15	8822.132	2	NC	1
69		16	max	.002	3	.006	2	.004	1	4.613e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	15	2.127e-5	15	7462.98	2	NC	1
71		17	max	.002	3	.007	2	.005	1	4.846e-4	1	NC	1	NC	2
72			min	-.002	2	-.009	3	0	15	2.236e-5	15	6414.837	2	8838.925	1
73		18	max	.002	3	.008	2	.006	1	5.08e-4	1	NC	1	NC	2
74			min	-.002	2	-.009	3	0	15	2.344e-5	15	5596.631	2	7714.121	1
75		19	max	.002	3	.009	2	.007	1	5.313e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	15	2.453e-5	15	4951.791	2	6855.301	1
77	M4	1	max	.002	1	.012	2	0	15	-3.628e-5	12	NC	1	NC	3
78			min	0	15	-.011	3	-.005	1	-8.638e-4	1	NC	1	3824.226	1
79		2	max	.002	1	.012	2	0	15	-3.628e-5	12	NC	1	NC	3
80			min	0	15	-.01	3	-.005	1	-8.638e-4	1	NC	1	4171.977	1
81		3	max	.002	1	.011	2	0	15	-3.628e-5	12	NC	1	NC	2
82			min	0	15	-.01	3	-.004	1	-8.638e-4	1	NC	1	4585.852	1
83		4	max	.001	1	.01	2	0	15	-3.628e-5	12	NC	1	NC	2
84			min	0	15	-.009	3	-.004	1	-8.638e-4	1	NC	1	5083.293	1
85		5	max	.001	1	.01	2	0	15	-3.628e-5	12	NC	1	NC	2
86			min	0	15	-.008	3	-.003	1	-8.638e-4	1	NC	1	5688.061	1
87		6	max	.001	1	.009	2	0	15	-3.628e-5	12	NC	1	NC	2
88			min	0	15	-.008	3	-.003	1	-8.638e-4	1	NC	1	6433.207	1
89		7	max	.001	1	.008	2	0	15	-3.628e-5	12	NC	1	NC	2
90			min	0	15	-.007	3	-.003	1	-8.638e-4	1	NC	1	7365.807	1
91		8	max	.001	1	.007	2	0	15	-3.628e-5	12	NC	1	NC	2
92			min	0	15	-.007	3	-.002	1	-8.638e-4	1	NC	1	8554.804	1
93		9	max	0	1	.007	2	0	15	-3.628e-5	12	NC	1	NC	1
94			min	0	15	-.006	3	-.002	1	-8.638e-4	1	NC	1	NC	1
95		10	max	0	1	.006	2	0	15	-3.628e-5	12	NC	1	NC	1
96			min	0	15	-.005	3	-.002	1	-8.638e-4	1	NC	1	NC	1
97		11	max	0	1	.005	2	0	15	-3.628e-5	12	NC	1	NC	1
98			min	0	15	-.005	3	-.001	1	-8.638e-4	1	NC	1	NC	1
99		12	max	0	1	.005	2	0	15	-3.628e-5	12	NC	1	NC	1
100			min	0	15	-.004	3	-.001	1	-8.638e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	15	-3.628e-5	12	NC	1	NC	1
102			min	0	15	-.004	3	0	1	-8.638e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	15	-3.628e-5	12	NC	1	NC	1
104			min	0	15	-.003	3	0	1	-8.638e-4	1	NC	1	NC	1
105		15	max	0	1	.003	2	0	15	-3.628e-5	12	NC	1	NC	1
106			min	0	15	-.002	3	0	1	-8.638e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	15	-3.628e-5	12	NC	1	NC	1
108			min	0	15	-.002	3	0	1	-8.638e-4	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-3.628e-5	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-8.638e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.628e-5	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-8.638e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.628e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-8.638e-4	1	NC	1	NC	1
115	M6	1	max	.008	1	.037	2	.005	1	3.83e-4	3	NC	3	NC	2
116			min	-.012	3	-.035	3	-.005	3	2.533e-7	10	1143.055	2	8850.968	1
117		2	max	.008	1	.035	2	.004	1	3.701e-4	3	NC	3	NC	2
118			min	-.011	3	-.033	3	-.004	3	-3.301e-7	10	1223.77	2	9564.729	1
119		3	max	.007	1	.032	2	.004	1	3.572e-4	3	NC	3	NC	1
120			min	-.011	3	-.031	3	-.004	3	-1.889e-6	2	1316.335	2	NC	1
121		4	max	.007	1	.03	2	.004	1	3.443e-4	3	NC	3	NC	1
122			min	-.01	3	-.029	3	-.004	3	-4.212e-6	2	1423.112	2	NC	1
123		5	max	.007	1	.027	2	.003	1	3.314e-4	3	NC	3	NC	1
124			min	-.009	3	-.027	3	-.004	3	-6.534e-6	2	1547.143	2	NC	1
125		6	max	.006	1	.025	2	.003	1	3.185e-4	3	NC	3	NC	1
126			min	-.009	3	-.026	3	-.003	3	-8.857e-6	2	1692.407	2	NC	1
127		7	max	.006	1	.023	2	.003	1	3.056e-4	3	NC	3	NC	1
128			min	-.008	3	-.024	3	-.003	3	-1.118e-5	2	1864.212	2	NC	1
129		8	max	.005	1	.021	2	.002	1	2.927e-4	3	NC	3	NC	1
130			min	-.007	3	-.022	3	-.003	3	-1.35e-5	2	2069.793	2	NC	1
131		9	max	.005	1	.018	2	.002	1	2.798e-4	3	NC	3	NC	1
132			min	-.007	3	-.02	3	-.002	3	-1.582e-5	2	2319.276	2	NC	1
133		10	max	.004	1	.016	2	.002	1	2.669e-4	3	NC	3	NC	1
134			min	-.006	3	-.018	3	-.002	3	-1.815e-5	2	2627.279	2	NC	1
135		11	max	.004	1	.014	2	.001	1	2.54e-4	3	NC	3	NC	1
136			min	-.005	3	-.016	3	-.002	3	-2.047e-5	2	3015.711	2	NC	1
137		12	max	.003	1	.012	2	.001	1	2.411e-4	3	NC	3	NC	1
138			min	-.005	3	-.014	3	-.001	3	-2.279e-5	2	3518.982	2	NC	1
139		13	max	.003	1	.01	2	0	1	2.282e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.001	3	-2.511e-5	2	4194.404	2	NC	1
141		14	max	.002	1	.008	2	0	1	2.153e-4	3	NC	3	NC	1
142			min	-.003	3	-.01	3	0	3	-2.778e-5	11	5145.083	2	NC	1
143		15	max	.002	1	.006	2	0	1	2.024e-4	3	NC	1	NC	1
144			min	-.003	3	-.008	3	0	3	-3.16e-5	11	6577.155	2	NC	1
145		16	max	.001	1	.005	2	0	1	1.895e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-3.613e-5	1	8971.465	2	NC	1
147		17	max	0	1	.003	2	0	1	1.766e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-4.537e-5	1	NC	1	NC	1
149		18	max	0	1	.002	2	0	1	1.637e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-5.462e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.508e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-6.387e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	3.02e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-7.181e-5	3	NC	1	NC	1
155		2	max	0	3	.002	2	0	3	2.668e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-5.242e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	2.316e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-3.302e-5	3	NC	1	NC	1
159		4	max	.001	3	.005	2	0	3	1.964e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.363e-5	3	NC	1	NC	1
161		5	max	.002	3	.006	2	.001	3	1.612e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	5.828e-7	15	7635.773	2	NC	1
163		6	max	.002	3	.008	2	.001	3	2.515e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	5.761e-7	15	6114.8	2	NC	1
165		7	max	.003	3	.009	2	.002	3	4.455e-5	3	NC	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.012	3	0	1	5.695e-7	15	5078.125	2	NC	1
167		8	max	.003	3	.011	2	.002	3	6.394e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	-.001	1	-2.346e-6	11	4320.701	2	NC	1
169		9	max	.003	3	.012	2	.002	3	8.333e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	-.001	1	-5.855e-6	11	3740.265	2	NC	1
171		10	max	.004	3	.014	2	.002	3	1.027e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	-.001	1	-9.364e-6	11	3280.095	2	NC	1
173		11	max	.004	3	.016	2	.002	3	1.221e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.287e-5	11	2906.126	2	NC	1
175		12	max	.005	3	.018	2	.002	3	1.415e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.002	1	-1.638e-5	11	2596.582	2	NC	1
177		13	max	.005	3	.02	2	.002	3	1.609e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.002	1	-1.989e-5	11	2336.809	2	NC	1
179		14	max	.005	3	.022	2	.002	3	1.803e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.002	1	-2.34e-5	11	2116.515	2	NC	1
181		15	max	.006	3	.024	2	.002	3	1.997e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.002	1	-2.691e-5	11	1928.205	2	NC	1
183		16	max	.006	3	.026	2	.002	3	2.191e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.002	1	-3.042e-5	11	1766.259	2	NC	1
185		17	max	.007	3	.028	2	.002	3	2.385e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.002	1	-3.393e-5	11	1626.354	2	NC	1
187		18	max	.007	3	.031	2	.002	3	2.579e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-3.744e-5	11	1505.101	2	NC	1
189		19	max	.008	3	.033	2	.002	3	2.773e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-4.095e-5	11	1399.8	2	NC	1
191	M8	1	max	.004	1	.043	2	.002	1	-3.523e-6	10	NC	1	NC	2
192			min	0	15	-.034	3	-.002	3	-2.223e-4	3	NC	1	7968.527	1
193		2	max	.004	1	.041	2	.002	1	-3.523e-6	10	NC	1	NC	2
194			min	0	15	-.032	3	-.001	3	-2.223e-4	3	NC	1	8687.836	1
195		3	max	.004	1	.038	2	.002	1	-3.523e-6	10	NC	1	NC	2
196			min	0	15	-.03	3	-.001	3	-2.223e-4	3	NC	1	9544.175	1
197		4	max	.004	1	.036	2	.002	1	-3.523e-6	10	NC	1	NC	1
198			min	0	15	-.029	3	-.001	3	-2.223e-4	3	NC	1	NC	1
199		5	max	.003	1	.033	2	.002	1	-3.523e-6	10	NC	1	NC	1
200			min	0	15	-.027	3	-.001	3	-2.223e-4	3	NC	1	NC	1
201		6	max	.003	1	.031	2	.001	1	-3.523e-6	10	NC	1	NC	1
202			min	0	15	-.025	3	0	3	-2.223e-4	3	NC	1	NC	1
203		7	max	.003	1	.029	2	.001	1	-3.523e-6	10	NC	1	NC	1
204			min	0	15	-.023	3	0	3	-2.223e-4	3	NC	1	NC	1
205		8	max	.003	1	.026	2	.001	1	-3.523e-6	10	NC	1	NC	1
206			min	0	15	-.021	3	0	3	-2.223e-4	3	NC	1	NC	1
207		9	max	.002	1	.024	2	0	1	-3.523e-6	10	NC	1	NC	1
208			min	0	15	-.019	3	0	3	-2.223e-4	3	NC	1	NC	1
209		10	max	.002	1	.022	2	0	1	-3.523e-6	10	NC	1	NC	1
210			min	0	15	-.017	3	0	3	-2.223e-4	3	NC	1	NC	1
211		11	max	.002	1	.019	2	0	1	-3.523e-6	10	NC	1	NC	1
212			min	0	15	-.015	3	0	3	-2.223e-4	3	NC	1	NC	1
213		12	max	.002	1	.017	2	0	1	-3.523e-6	10	NC	1	NC	1
214			min	0	15	-.013	3	0	3	-2.223e-4	3	NC	1	NC	1
215		13	max	.001	1	.014	2	0	1	-3.523e-6	10	NC	1	NC	1
216			min	0	15	-.011	3	0	3	-2.223e-4	3	NC	1	NC	1
217		14	max	.001	1	.012	2	0	1	-3.523e-6	10	NC	1	NC	1
218			min	0	15	-.01	3	0	3	-2.223e-4	3	NC	1	NC	1
219		15	max	0	1	.01	2	0	1	-3.523e-6	10	NC	1	NC	1
220			min	0	15	-.008	3	0	3	-2.223e-4	3	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-3.523e-6	10	NC	1	NC	1
222			min	0	15	-.006	3	0	3	-2.223e-4	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.005	2	0	1	-3.523e-6	10	NC	1	NC	1
224			min	0	15	-.004	3	0	3	-2.223e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-3.523e-6	10	NC	1	NC	1
226			min	0	15	-.002	3	0	3	-2.223e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-3.523e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.223e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.01	2	0	3	1.003e-3	1	NC	3	NC	1
230			min	-.003	3	-.011	3	-.002	1	-3.893e-4	3	4085.38	2	NC	1
231		2	max	.003	1	.009	2	0	3	9.516e-4	1	NC	3	NC	1
232			min	-.003	3	-.01	3	-.002	1	-3.759e-4	3	4472.875	2	NC	1
233		3	max	.002	1	.009	2	0	3	9.003e-4	1	NC	1	NC	1
234			min	-.003	3	-.01	3	-.002	1	-3.626e-4	3	4936.419	2	NC	1
235		4	max	.002	1	.008	2	0	3	8.491e-4	1	NC	1	NC	1
236			min	-.003	3	-.01	3	-.002	1	-3.493e-4	3	5494.839	2	NC	1
237		5	max	.002	1	.007	2	0	3	7.979e-4	1	NC	1	NC	1
238			min	-.003	3	-.009	3	-.002	1	-3.359e-4	3	6173.372	2	NC	1
239		6	max	.002	1	.006	2	0	3	7.467e-4	1	NC	1	NC	1
240			min	-.002	3	-.009	3	-.002	1	-3.226e-4	3	7006.401	2	NC	1
241		7	max	.002	1	.005	2	0	3	6.955e-4	1	NC	1	NC	1
242			min	-.002	3	-.008	3	-.002	1	-3.092e-4	3	8041.669	2	NC	1
243		8	max	.002	1	.005	2	0	3	6.443e-4	1	NC	1	NC	1
244			min	-.002	3	-.008	3	-.001	1	-2.959e-4	3	9346.956	2	NC	1
245		9	max	.002	1	.004	2	0	3	5.93e-4	1	NC	1	NC	1
246			min	-.002	3	-.007	3	-.001	1	-2.825e-4	3	NC	1	NC	1
247		10	max	.001	1	.003	2	0	3	5.418e-4	1	NC	1	NC	1
248			min	-.002	3	-.007	3	-.001	1	-2.692e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	4.906e-4	1	NC	1	NC	1
250			min	-.002	3	-.006	3	0	1	-2.559e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.394e-4	1	NC	1	NC	1
252			min	-.001	3	-.005	3	0	1	-2.425e-4	3	NC	1	NC	1
253		13	max	0	1	.002	2	0	3	3.882e-4	1	NC	1	NC	1
254			min	-.001	3	-.005	3	0	1	-2.292e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.37e-4	1	NC	1	NC	1
256			min	0	3	-.004	3	0	1	-2.158e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.857e-4	1	NC	1	NC	1
258			min	0	3	-.003	3	0	1	-2.025e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.345e-4	1	NC	1	NC	1
260			min	0	3	-.003	3	0	1	-1.891e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.833e-4	1	NC	1	NC	1
262			min	0	3	-.002	3	0	1	-1.758e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.321e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.625e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	8.087e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.491e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	7.133e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.968e-5	1	NC	1	NC	1
269		2	max	0	3	0	2	0	1	5.089e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-8.247e-5	1	NC	1	NC	1
271		3	max	0	3	0	2	0	11	3.044e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.253e-4	1	NC	1	NC	1
273		4	max	0	3	0	2	0	10	9.993e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.68e-4	1	NC	1	NC	1
275		5	max	0	3	0	2	0	10	-7.182e-6	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	3	-2.108e-4	1	NC	1	NC	1
277		6	max	0	3	0	2	0	10	-1.152e-5	15	NC	1	NC	1
278			min	0	2	-.005	3	-.001	3	-2.536e-4	1	NC	1	NC	1
279		7	max	0	3	0	2	0	10	-1.36e-5	15	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	3	-2.964e-4	1	NC	1	NC	1
281		8	max	0	3	0	2	0	15	-1.569e-5	15	NC	1	NC	1
282			min	0	2	-.006	3	-.002	1	-3.392e-4	1	NC	1	NC	1
283		9	max	.001	3	.001	2	0	15	-1.778e-5	15	NC	1	NC	1
284			min	-.001	2	-.007	3	-.003	1	-3.82e-4	1	NC	1	NC	1
285		10	max	.001	3	.002	2	0	15	-1.987e-5	15	NC	1	NC	1
286			min	-.001	2	-.007	3	-.004	1	-4.248e-4	1	NC	1	NC	1
287		11	max	.001	3	.002	2	0	15	-2.196e-5	15	NC	1	NC	2
288			min	-.001	2	-.008	3	-.005	1	-4.676e-4	1	NC	1	9633.364	1
289		12	max	.001	3	.003	2	0	15	-2.405e-5	15	NC	1	NC	2
290			min	-.001	2	-.008	3	-.006	1	-5.104e-4	1	NC	1	8003.542	1
291		13	max	.002	3	.004	2	0	15	-2.614e-5	15	NC	1	NC	2
292			min	-.002	2	-.008	3	-.007	1	-5.532e-4	1	NC	1	6802.376	1
293		14	max	.002	3	.004	2	0	15	-2.823e-5	15	NC	1	NC	2
294			min	-.002	2	-.009	3	-.008	1	-5.959e-4	1	NC	1	5891.496	1
295		15	max	.002	3	.005	2	0	15	-3.031e-5	15	NC	1	NC	2
296			min	-.002	2	-.009	3	-.009	1	-6.387e-4	1	8834.619	2	5184.763	1
297		16	max	.002	3	.006	2	0	15	-3.24e-5	15	NC	1	NC	2
298			min	-.002	2	-.009	3	-.01	1	-6.815e-4	1	7472.624	2	4626.214	1
299		17	max	.002	3	.007	2	0	15	-3.449e-5	15	NC	1	NC	2
300			min	-.002	2	-.009	3	-.011	1	-7.243e-4	1	6422.487	2	4178.191	1
301		18	max	.002	3	.008	2	0	15	-3.658e-5	15	NC	1	NC	2
302			min	-.002	2	-.009	3	-.012	1	-7.671e-4	1	5602.855	2	3814.603	1
303		19	max	.002	3	.009	2	0	15	-3.867e-5	15	NC	3	NC	3
304			min	-.002	2	-.009	3	-.013	1	-8.099e-4	1	4956.978	2	3516.931	1
305	M12	1	max	.002	1	.012	2	.011	1	7.81e-4	1	NC	1	NC	3
306			min	0	15	-.011	3	0	15	3.82e-5	15	NC	1	1773.758	1
307		2	max	.002	1	.012	2	.01	1	7.81e-4	1	NC	1	NC	3
308			min	0	15	-.01	3	0	15	3.82e-5	15	NC	1	1934.434	1
309		3	max	.002	1	.011	2	.009	1	7.81e-4	1	NC	1	NC	3
310			min	0	15	-.01	3	0	15	3.82e-5	15	NC	1	2125.691	1
311		4	max	.001	1	.01	2	.008	1	7.81e-4	1	NC	1	NC	3
312			min	0	15	-.009	3	0	15	3.82e-5	15	NC	1	2355.591	1
313		5	max	.001	1	.009	2	.007	1	7.81e-4	1	NC	1	NC	3
314			min	0	15	-.008	3	0	15	3.82e-5	15	NC	1	2635.115	1
315		6	max	.001	1	.009	2	.006	1	7.81e-4	1	NC	1	NC	3
316			min	0	15	-.008	3	0	15	3.82e-5	15	NC	1	2979.537	1
317		7	max	.001	1	.008	2	.006	1	7.81e-4	1	NC	1	NC	3
318			min	0	15	-.007	3	0	15	3.82e-5	15	NC	1	3410.614	1
319		8	max	.001	1	.007	2	.005	1	7.81e-4	1	NC	1	NC	2
320			min	0	15	-.007	3	0	15	3.82e-5	15	NC	1	3960.208	1
321		9	max	0	1	.007	2	.004	1	7.81e-4	1	NC	1	NC	2
322			min	0	15	-.006	3	0	15	3.82e-5	15	NC	1	4676.512	1
323		10	max	0	1	.006	2	.003	1	7.81e-4	1	NC	1	NC	2
324			min	0	15	-.005	3	0	15	3.82e-5	15	NC	1	5635.296	1
325		11	max	0	1	.005	2	.003	1	7.81e-4	1	NC	1	NC	2
326			min	0	15	-.005	3	0	15	3.82e-5	15	NC	1	6961.367	1
327		12	max	0	1	.005	2	.002	1	7.81e-4	1	NC	1	NC	2
328			min	0	15	-.004	3	0	15	3.82e-5	15	NC	1	8872.452	1
329		13	max	0	1	.004	2	.002	1	7.81e-4	1	NC	1	NC	1
330			min	0	15	-.004	3	0	15	3.82e-5	15	NC	1	NC	1
331		14	max	0	1	.003	2	.001	1	7.81e-4	1	NC	1	NC	1
332			min	0	15	-.003	3	0	15	3.82e-5	15	NC	1	NC	1
333		15	max	0	1	.003	2	0	1	7.81e-4	1	NC	1	NC	1
334			min	0	15	-.002	3	0	15	3.82e-5	15	NC	1	NC	1
335		16	max	0	1	.002	2	0	1	7.81e-4	1	NC	1	NC	1
336			min	0	15	-.002	3	0	15	3.82e-5	15	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	7.81e-4	1	NC	1	NC	1
338			min	0	15	-.001	3	0	15	3.82e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	7.81e-4	1	NC	1	NC	1
340			min	0	15	0	3	0	15	3.82e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	7.81e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.82e-5	15	NC	1	NC	1
343	M1	1	max	.009	3	.026	3	.003	3	1.55e-2	1	NC	1	NC	1
344			min	-.009	2	-.023	2	-.005	1	-2.113e-2	3	NC	1	NC	1
345		2	max	.009	3	.016	3	.002	3	7.425e-3	2	NC	4	NC	2
346			min	-.009	2	-.014	2	-.01	1	-1.047e-2	3	4999.263	2	8995.44	1
347		3	max	.009	3	.007	3	.001	3	-5.348e-6	12	NC	4	NC	2
348			min	-.009	2	-.005	2	-.013	1	-6.829e-4	1	2565.875	2	5459.766	1
349		4	max	.009	3	.003	1	0	3	-5.601e-8	3	NC	4	NC	2
350			min	-.009	2	-.002	3	-.015	1	-5.913e-4	1	1794.669	2	4523.391	1
351		5	max	.009	3	.01	2	0	3	5.54e-6	3	NC	4	NC	2
352			min	-.009	2	-.008	3	-.015	1	-4.997e-4	1	1422.065	2	4350.872	1
353		6	max	.009	3	.015	2	0	3	1.114e-5	3	NC	5	NC	2
354			min	-.009	2	-.014	3	-.014	1	-4.082e-4	1	1209.938	2	4667.126	1
355		7	max	.009	3	.02	2	0	3	1.673e-5	3	NC	5	NC	2
356			min	-.009	2	-.018	3	-.013	1	-3.166e-4	1	1080.054	2	5579.716	1
357		8	max	.009	3	.024	2	0	3	2.233e-5	3	NC	5	NC	2
358			min	-.009	2	-.021	3	-.01	1	-2.25e-4	1	999.845	2	7719.654	1
359		9	max	.009	3	.026	2	0	3	2.792e-5	3	NC	5	NC	1
360			min	-.009	2	-.023	3	-.007	1	-1.334e-4	1	954.124	2	NC	1
361		10	max	.009	3	.027	2	0	3	3.352e-5	3	NC	5	NC	1
362			min	-.009	2	-.023	3	-.004	1	-4.186e-5	1	935.961	2	NC	1
363		11	max	.009	3	.026	2	0	3	4.971e-5	1	NC	5	NC	1
364			min	-.009	2	-.022	3	-.001	1	2.696e-6	15	943.468	2	NC	1
365		12	max	.009	3	.025	2	.002	1	1.413e-4	1	NC	5	NC	2
366			min	-.009	2	-.021	3	0	15	6.866e-6	15	979.128	2	8410.043	1
367		13	max	.009	3	.021	2	.004	1	2.329e-4	1	NC	5	NC	2
368			min	-.009	2	-.018	3	0	15	1.104e-5	15	1051.144	2	5907.225	1
369		14	max	.009	3	.016	2	.006	1	3.244e-4	1	NC	4	NC	2
370			min	-.009	2	-.013	3	0	15	1.521e-5	15	1178.242	2	4870.803	1
371		15	max	.009	3	.01	2	.007	1	4.16e-4	1	NC	4	NC	2
372			min	-.009	2	-.008	3	0	15	1.938e-5	15	1404.182	2	4501.764	1
373		16	max	.009	3	.002	1	.007	1	4.78e-4	1	NC	4	NC	2
374			min	-.009	2	-.002	3	0	15	2.222e-5	15	1850.769	2	4651.108	1
375		17	max	.009	3	.006	3	.005	1	3.322e-5	3	NC	4	NC	2
376			min	-.009	2	-.008	2	0	15	-1.63e-4	1	2657.803	1	5590.61	1
377		18	max	.009	3	.014	3	.002	1	1.111e-2	2	NC	2	NC	2
378			min	-.009	2	-.019	2	0	15	-5.251e-3	3	5138.037	1	9185.523	1
379		19	max	.009	3	.022	3	0	3	2.249e-2	2	NC	1	NC	1
380			min	-.009	2	-.031	2	-.003	1	-1.063e-2	3	5933.943	2	NC	1
381	M5	1	max	.03	3	.086	3	.003	3	2.364e-6	3	NC	1	NC	1
382			min	-.033	2	-.078	2	-.006	1	4.16e-8	10	3584.04	3	NC	1
383		2	max	.03	3	.052	3	.004	3	1.071e-4	3	NC	5	NC	1
384			min	-.033	2	-.047	2	-.005	1	-5.782e-5	1	1494.373	2	NC	1
385		3	max	.03	3	.021	3	.005	3	2.098e-4	3	NC	5	NC	1
386			min	-.033	2	-.017	2	-.005	1	-1.152e-4	1	766.545	2	NC	1
387		4	max	.03	3	.009	2	.005	3	2.018e-4	3	NC	5	NC	1
388			min	-.033	2	-.005	3	-.004	1	-1.105e-4	1	535.613	2	NC	1
389		5	max	.03	3	.032	2	.006	3	1.937e-4	3	NC	5	NC	1
390			min	-.033	2	-.027	3	-.004	1	-1.057e-4	1	423.995	2	NC	1
391		6	max	.029	3	.052	2	.006	3	1.857e-4	3	NC	5	NC	1
392			min	-.033	2	-.045	3	-.004	1	-1.01e-4	1	360.419	2	NC	1
393		7	max	.029	3	.067	2	.006	3	1.777e-4	3	NC	15	NC	1







Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.009	3	.006	3	0	15	7.608e-5	3	NC	4	NC	2
452			min	-.009	2	-.008	2	-.011	1	-2.49e-4	1	2657.585	1	5436.397	1
453		18	max	.009	3	.014	3	0	15	5.307e-3	3	NC	2	NC	2
454			min	-.009	2	-.019	2	-.007	1	-1.119e-2	2	5137.63	1	8999.156	1
455		19	max	.009	3	.023	3	0	3	1.063e-2	3	NC	1	NC	1
456			min	-.009	2	-.031	2	-.002	1	-2.249e-2	2	5957.378	2	NC	1
457	M13	1	max	.006	1	.026	3	.009	3	3.872e-3	3	NC	1	NC	1
458			min	-.002	3	-.023	2	-.009	2	-3.559e-3	2	NC	1	NC	1
459		2	max	.006	1	.21	3	.033	1	4.848e-3	3	NC	5	NC	2
460			min	-.002	3	-.156	2	-.002	10	-4.487e-3	2	881.973	3	4324.848	1
461		3	max	.006	1	.36	3	.085	1	5.825e-3	3	NC	5	NC	3
462			min	-.002	3	-.265	1	.004	10	-5.415e-3	2	484.956	3	1818.28	1
463		4	max	.006	1	.455	3	.128	1	6.801e-3	3	NC	5	NC	3
464			min	-.002	3	-.335	1	.006	15	-6.343e-3	2	377.924	3	1223.427	1
465		5	max	.006	1	.483	3	.149	1	7.778e-3	3	NC	5	NC	3
466			min	-.002	3	-.357	1	.007	15	-7.272e-3	2	354.578	3	1058.431	1
467		6	max	.006	1	.446	3	.141	1	8.754e-3	3	NC	5	NC	3
468			min	-.002	3	-.331	2	.005	10	-8.2e-3	2	385.763	3	1118.81	1
469		7	max	.006	1	.357	3	.105	1	9.731e-3	3	NC	5	NC	3
470			min	-.002	3	-.269	2	-.001	10	-9.128e-3	2	489.549	3	1478.816	1
471		8	max	.006	1	.241	3	.053	1	1.071e-2	3	NC	5	NC	2
472			min	-.002	3	-.188	2	-.009	10	-1.006e-2	2	754.547	3	2824.413	1
473		9	max	.006	1	.134	3	.027	3	1.168e-2	3	NC	4	NC	1
474			min	-.002	3	-.113	2	-.023	2	-1.098e-2	2	1499.49	3	9017.927	3
475		10	max	.006	1	.086	3	.03	3	1.266e-2	3	NC	4	NC	4
476			min	-.003	3	-.078	2	-.033	2	-1.191e-2	2	2721.808	3	6721.744	2
477		11	max	.005	1	.134	3	.034	3	1.168e-2	3	NC	4	NC	1
478			min	-.003	3	-.113	2	-.023	2	-1.098e-2	2	1499.488	3	6679.248	3
479		12	max	.005	1	.241	3	.056	1	1.071e-2	3	NC	5	NC	2
480			min	-.003	3	-.188	2	-.009	10	-1.006e-2	2	754.546	3	2678.642	1
481		13	max	.005	1	.357	3	.109	1	9.734e-3	3	NC	5	NC	5
482			min	-.003	3	-.269	2	-.001	10	-9.128e-3	2	489.548	3	1432.318	1
483		14	max	.005	1	.446	3	.144	1	8.759e-3	3	NC	5	NC	5
484			min	-.003	3	-.331	2	.005	10	-8.2e-3	2	385.763	3	1092.819	1
485		15	max	.005	1	.483	3	.152	1	7.783e-3	3	NC	5	NC	5
486			min	-.003	3	-.357	1	.007	15	-7.272e-3	2	354.577	3	1038.509	1
487		16	max	.005	1	.455	3	.13	1	6.808e-3	3	NC	5	NC	5
488			min	-.003	3	-.335	1	.006	15	-6.344e-3	2	377.924	3	1203.561	1
489		17	max	.005	1	.36	3	.086	1	5.833e-3	3	NC	5	NC	3
490			min	-.003	3	-.265	1	.003	10	-5.416e-3	2	484.955	3	1791.248	1
491		18	max	.005	1	.21	3	.034	1	4.857e-3	3	NC	5	NC	2
492			min	-.003	3	-.156	2	-.002	10	-4.488e-3	2	881.972	3	4260.555	1
493		19	max	.005	1	.026	3	.009	3	3.882e-3	3	NC	1	NC	1
494			min	-.003	3	-.023	2	-.009	2	-3.561e-3	2	NC	1	NC	1
495	M16	1	max	.002	1	.023	3	.009	3	4.567e-3	2	NC	1	NC	1
496			min	0	3	-.031	2	-.009	2	-3.247e-3	3	NC	1	NC	1
497		2	max	.002	1	.115	3	.034	1	5.77e-3	2	NC	5	NC	2
498			min	0	3	-.227	2	-.002	10	-4.049e-3	3	828.354	2	4174.102	1
499		3	max	.002	1	.191	3	.087	1	6.974e-3	2	NC	5	NC	3
500			min	0	3	-.387	2	.004	10	-4.851e-3	3	455.082	2	1772.093	1
501		4	max	.002	1	.24	3	.131	1	8.178e-3	2	NC	5	NC	5
502			min	0	3	-.489	2	.006	15	-5.653e-3	3	354.088	2	1196.975	1
503		5	max	.002	1	.257	3	.152	1	9.381e-3	2	NC	5	NC	5
504			min	0	3	-.52	2	.007	15	-6.456e-3	3	331.336	2	1036.864	1
505		6	max	.002	1	.242	3	.143	1	1.058e-2	2	NC	5	NC	5
506			min	0	3	-.483	2	.005	10	-7.258e-3	3	358.828	2	1095.268	1
507		7	max	.002	1	.201	3	.108	1	1.179e-2	2	NC	5	NC	5



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.39	2	0	10	-8.06e-3	3	451.435	2	1442.849	1
509	8	max	.002	1	.146	3	.055	1	1.299e-2	2	NC	5	NC	2
510		min	0	3	-.268	2	-.009	10	-8.862e-3	3	682.704	2	2726.62	1
511	9	max	.002	1	.096	3	.031	3	1.42e-2	2	NC	4	NC	1
512		min	0	3	-.157	2	-.023	2	-9.665e-3	3	1291.232	2	7295.543	3
513	10	max	.002	1	.073	3	.028	3	1.54e-2	2	NC	4	NC	4
514		min	0	3	-.106	2	-.033	2	-1.047e-2	3	2173.572	2	6776.662	2
515	11	max	.003	1	.096	3	.028	3	1.42e-2	2	NC	4	NC	1
516		min	0	3	-.157	2	-.022	2	-9.664e-3	3	1291.232	2	8604.349	3
517	12	max	.003	1	.146	3	.054	1	1.299e-2	2	NC	5	NC	2
518		min	0	3	-.268	2	-.009	10	-8.86e-3	3	682.704	2	2761.48	1
519	13	max	.003	1	.201	3	.106	1	1.179e-2	2	NC	5	NC	5
520		min	0	3	-.39	2	0	10	-8.057e-3	3	451.435	2	1459.257	1
521	14	max	.003	1	.242	3	.142	1	1.059e-2	2	NC	5	NC	5
522		min	0	3	-.483	2	.005	10	-7.253e-3	3	358.828	2	1108.261	1
523	15	max	.003	1	.257	3	.15	1	9.383e-3	2	NC	5	NC	5
524		min	0	3	-.52	2	.007	15	-6.45e-3	3	331.336	2	1050.753	1
525	16	max	.003	1	.24	3	.129	1	8.179e-3	2	NC	5	NC	3
526		min	0	3	-.489	2	.006	15	-5.646e-3	3	354.088	2	1216.358	1
527	17	max	.003	1	.191	3	.085	1	6.976e-3	2	NC	5	NC	3
528		min	0	3	-.387	2	.004	10	-4.843e-3	3	455.082	2	1809.895	1
529	18	max	.003	1	.115	3	.033	1	5.772e-3	2	NC	5	NC	2
530		min	0	3	-.227	2	-.002	10	-4.04e-3	3	828.355	2	4309.978	1
531	19	max	.003	1	.022	3	.009	3	4.569e-3	2	NC	1	NC	1
532		min	0	3	-.031	2	-.009	2	-3.236e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.003e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-5.848e-5	2	NC	1	NC	1
535	2	max	0	3	-.003	15	.001	1	9.092e-4	3	NC	3	NC	1
536		min	0	2	-.015	4	0	3	-5.832e-4	2	6227.544	4	NC	1
537	3	max	0	3	-.007	15	.004	1	1.418e-3	3	NC	15	NC	1
538		min	0	2	-.029	4	-.004	3	-1.108e-3	2	3168.984	4	NC	1
539	4	max	0	3	-.01	15	.008	1	1.927e-3	3	9248.97	15	NC	4
540		min	0	2	-.042	4	-.008	3	-1.633e-3	2	2174.108	4	6502.015	3
541	5	max	0	3	-.013	15	.012	1	2.436e-3	3	7217.065	15	NC	4
542		min	0	2	-.054	4	-.013	3	-2.157e-3	2	1696.479	4	4289.833	3
543	6	max	0	3	-.015	15	.017	1	2.945e-3	3	6073.923	15	NC	4
544		min	0	2	-.064	4	-.019	3	-2.682e-3	2	1427.766	4	3134.862	3
545	7	max	0	3	-.017	15	.022	1	3.453e-3	3	5386.474	15	NC	4
546		min	0	2	-.073	4	-.025	3	-3.207e-3	2	1266.171	4	2457.118	3
547	8	max	0	3	-.018	15	.027	1	3.962e-3	3	4973.901	15	NC	4
548		min	0	2	-.079	4	-.031	3	-3.731e-3	2	1169.19	4	2030.035	3
549	9	max	0	3	-.019	15	.032	1	4.471e-3	3	4751.83	15	NC	4
550		min	0	2	-.082	4	-.036	3	-4.256e-3	2	1116.989	4	1750.123	3
551	10	max	0	3	-.02	15	.035	1	4.98e-3	3	4681.577	15	NC	4
552		min	0	2	-.084	4	-.04	3	-4.781e-3	2	1100.475	4	1565.265	3
553	11	max	0	3	-.019	15	.037	1	5.489e-3	3	4751.83	15	NC	5
554		min	-.001	2	-.083	4	-.043	3	-5.305e-3	2	1116.989	4	1448.066	3
555	12	max	0	3	-.019	15	.038	1	5.998e-3	3	4973.901	15	NC	5
556		min	-.001	2	-.079	4	-.043	3	-5.83e-3	2	1169.19	4	1384.968	3
557	13	max	.001	3	-.017	15	.037	1	6.507e-3	3	5386.474	15	NC	5
558		min	-.001	2	-.073	4	-.042	3	-6.355e-3	2	1266.171	4	1372.142	3
559	14	max	.001	3	-.015	15	.034	1	7.016e-3	3	6073.923	15	NC	4
560		min	-.001	2	-.065	4	-.038	3	-6.88e-3	2	1427.766	4	1415.723	3
561	15	max	.001	3	-.013	15	.028	1	7.524e-3	3	7217.065	15	NC	4
562		min	-.001	2	-.055	4	-.031	3	-7.404e-3	2	1696.479	4	1537.833	3
563	16	max	.001	3	-.01	15	.019	1	8.033e-3	3	9248.97	15	NC	4
564		min	-.002	2	-.043	4	-.02	3	-7.929e-3	2	2174.108	4	1798.378	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	.001	3	-.007	15	.008	1	8.542e-3	3	NC	15	NC	4
566		min	-.002	2	-.03	4	-.005	3	-8.454e-3	2	3168.984	4	2385.18	3
567	18	max	.001	3	-.004	15	.013	3	9.051e-3	3	NC	3	NC	4
568		min	-.002	2	-.016	4	-.016	2	-8.978e-3	2	6227.544	4	4248.211	3
569	19	max	.002	3	.006	2	.037	3	9.56e-3	3	NC	1	NC	1
570		min	-.002	2	-.003	9	-.037	2	-9.503e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.011	3	2.8e-3	3	NC	1	NC	1
572		min	-.002	3	-.002	9	-.011	2	-2.631e-3	2	NC	1	NC	1
573	2	max	0	10	-.004	15	.004	9	2.69e-3	3	NC	3	NC	1
574		min	-.001	3	-.015	4	-.003	2	-2.517e-3	2	6227.544	4	NC	1
575	3	max	0	10	-.007	15	.01	1	2.58e-3	3	NC	15	NC	4
576		min	-.001	3	-.03	4	-.003	3	-2.402e-3	2	3168.984	4	5800.512	1
577	4	max	0	10	-.01	15	.016	1	2.471e-3	3	9248.97	15	NC	4
578		min	-.001	3	-.043	4	-.008	3	-2.288e-3	2	2174.108	4	4405.004	1
579	5	max	0	10	-.013	15	.019	1	2.361e-3	3	7217.065	15	NC	4
580		min	-.001	3	-.055	4	-.011	3	-2.174e-3	2	1696.479	4	3797.711	1
581	6	max	0	10	-.015	15	.021	1	2.251e-3	3	6073.923	15	NC	4
582		min	-.001	3	-.065	4	-.013	3	-2.059e-3	2	1427.766	4	3529.073	1
583	7	max	0	10	-.017	15	.022	1	2.141e-3	3	5386.474	15	NC	4
584		min	-.001	3	-.073	4	-.014	3	-1.945e-3	2	1266.171	4	3457.782	1
585	8	max	0	10	-.018	15	.022	1	2.032e-3	3	4973.901	15	NC	4
586		min	0	3	-.079	4	-.014	3	-1.83e-3	2	1169.19	4	3534.826	1
587	9	max	0	10	-.019	15	.021	1	1.922e-3	3	4751.83	15	NC	4
588		min	0	3	-.082	4	-.013	3	-1.716e-3	2	1116.989	4	3752.255	1
589	10	max	0	10	-.02	15	.019	1	1.812e-3	3	4681.577	15	NC	4
590		min	0	3	-.084	4	-.012	3	-1.601e-3	2	1100.475	4	4130.866	1
591	11	max	0	10	-.019	15	.017	1	1.703e-3	3	4751.83	15	NC	4
592		min	0	3	-.082	4	-.01	3	-1.487e-3	2	1116.989	4	4724.193	1
593	12	max	0	10	-.018	15	.014	1	1.593e-3	3	4973.901	15	NC	4
594		min	0	3	-.079	4	-.008	3	-1.372e-3	2	1169.19	4	5638.519	1
595	13	max	0	10	-.017	15	.011	1	1.483e-3	3	5386.474	15	NC	4
596		min	0	3	-.072	4	-.006	3	-1.258e-3	2	1266.171	4	7084.075	1
597	14	max	0	10	-.015	15	.008	1	1.374e-3	3	6073.923	15	NC	2
598		min	0	3	-.064	4	-.004	3	-1.143e-3	2	1427.766	4	9509.054	1
599	15	max	0	10	-.013	15	.005	1	1.264e-3	3	7217.065	15	NC	1
600		min	0	3	-.054	4	-.002	3	-1.029e-3	2	1696.479	4	NC	1
601	16	max	0	10	-.01	15	.003	1	1.154e-3	3	9248.97	15	NC	1
602		min	0	3	-.042	4	0	3	-9.141e-4	2	2174.108	4	NC	1
603	17	max	0	10	-.007	15	.001	9	1.044e-3	3	NC	15	NC	1
604		min	0	3	-.029	4	0	2	-7.996e-4	2	3168.984	4	NC	1
605	18	max	0	10	-.003	15	0	3	9.348e-4	3	NC	3	NC	1
606		min	0	3	-.015	4	0	2	-6.851e-4	2	6227.544	4	NC	1
607	19	max	0	1	0	1	0	1	8.251e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.706e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

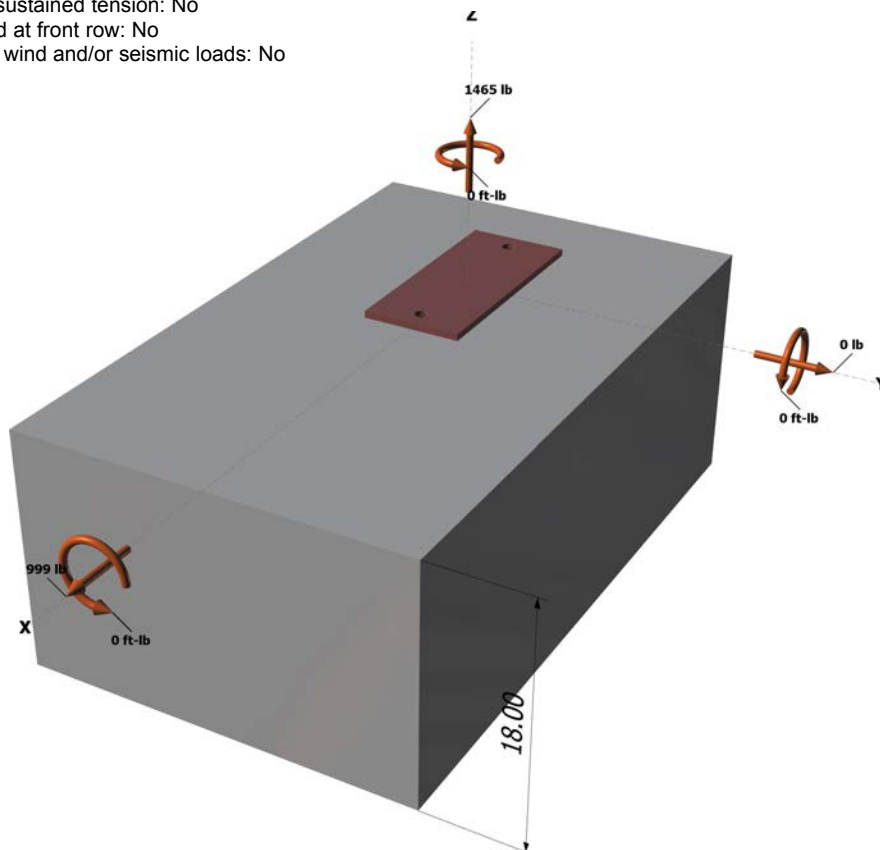
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
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- Refer to manufacturer's product literature for hole cleaning and installation instructions.