

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	90 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 12.72 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	135 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.767 k-ft
$M_z$ =	0.418 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>100%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.817 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.876 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>83%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.409 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>9%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.615 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>36%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.002 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>35%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

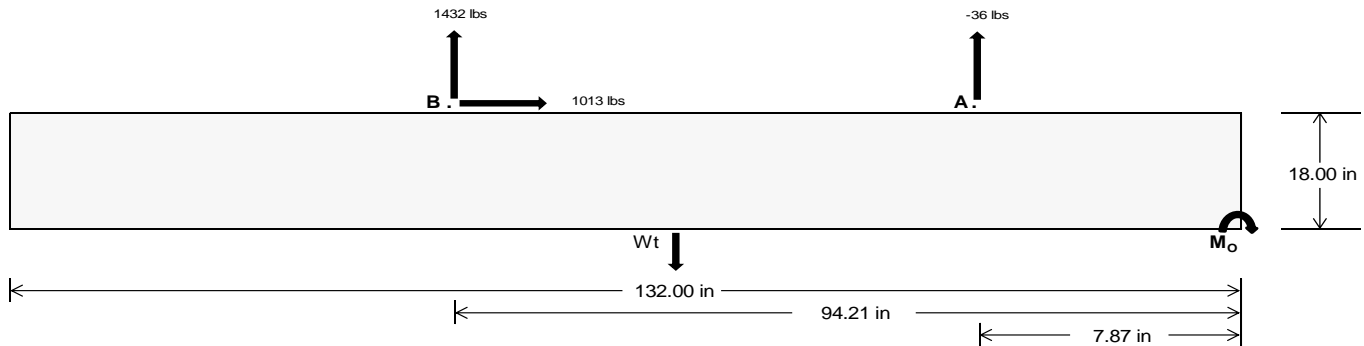
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>102.21</b>	<b>5969.28</b>	k
Compressive Load =	<b>3131.31</b>	<b>4789.87</b>	k
Lateral Load =	<b>19.65</b>	<b>4214.71</b>	k
Moment (Weak Axis) =	<b>0.04</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 152861.7$  in-lbs  
Resisting Force Required = 2316.09 lbs  
S.F. = 1.67  
Weight Required = 3860.14 lbs  
Minimum Width = 30 in  
Weight Provided = 5981.25 lbs

### Sliding

Force = 1012.70 lbs  
Friction = 0.4  
Weight Required = 2531.76 lbs  
Resisting Weight = 5981.25 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 1012.70 lbs  
Cohesion = 130 psf  
Area = 27.50 ft<sup>2</sup>  
Resisting = 2990.63 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 30in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 30in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 30in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.5 \text{ ft}) =$

Ballast Width			
30 in	31 in	32 in	33 in
5981 lbs	6181 lbs	6380 lbs	6579 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in	30 in	31 in	32 in	33 in
$F_A$	1205 lbs	1205 lbs	1205 lbs	1205 lbs	1045 lbs	1045 lbs	1045 lbs	1045 lbs	1540 lbs	1540 lbs	1540 lbs	1540 lbs	71 lbs	71 lbs	71 lbs	71 lbs
$F_B$	1088 lbs	1088 lbs	1088 lbs	1088 lbs	2111 lbs	2111 lbs	2111 lbs	2111 lbs	2267 lbs	2267 lbs	2267 lbs	2267 lbs	-2864 lbs	-2864 lbs	-2864 lbs	-2864 lbs
$F_V$	200 lbs	200 lbs	200 lbs	200 lbs	1861 lbs	1861 lbs	1861 lbs	1861 lbs	1523 lbs	1523 lbs	1523 lbs	1523 lbs	-2025 lbs	-2025 lbs	-2025 lbs	-2025 lbs
$P_{total}$	8274 lbs	8474 lbs	8673 lbs	8872 lbs	9138 lbs	9337 lbs	9537 lbs	9736 lbs	9789 lbs	9988 lbs	10187 lbs	10387 lbs	796 lbs	916 lbs	1035 lbs	1155 lbs
$M$	3576 lbs-ft	3576 lbs-ft	3576 lbs-ft	3576 lbs-ft	2891 lbs-ft	2891 lbs-ft	2891 lbs-ft	2891 lbs-ft	4414 lbs-ft	4414 lbs-ft	4414 lbs-ft	4414 lbs-ft	4040 lbs-ft	4040 lbs-ft	4040 lbs-ft	4040 lbs-ft
$e$	0.43 ft	0.42 ft	0.41 ft	0.40 ft	0.32 ft	0.31 ft	0.30 ft	0.30 ft	0.45 ft	0.44 ft	0.43 ft	0.42 ft	5.08 ft	4.41 ft	3.90 ft	3.50 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	230.0 psf	229.6 psf	229.2 psf	228.8 psf	274.9 psf	273.1 psf	271.4 psf	269.7 psf	268.4 psf	266.8 psf	265.2 psf	263.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	371.8 psf	366.8 psf	362.2 psf	357.8 psf	389.6 psf	384.1 psf	378.9 psf	374.0 psf	443.5 psf	436.2 psf	429.4 psf	422.9 psf	500.7 psf	217.3 psf	162.1 psf	139.9 psf

Maximum Bearing Pressure = 501 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 30in wide x 18in tall ballast foundation for an acceptable bearing pressure.

### Weak Side Design

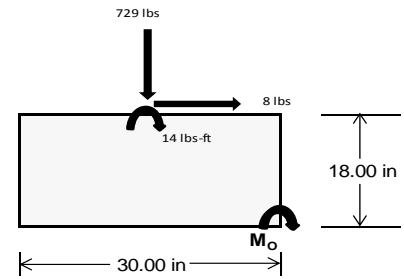
#### Overturning Check

$M_o = 885.3 \text{ ft-lbs}$   
 Resisting Force Required = 708.25 lbs  
 S.F. = 1.67  
 Weight Required = 1180.41 lbs  
 Minimum Width = 30 in  
 Weight Provided = 5981.25 lbs

*A minimum 132in long x 30in wide x 18in tall ballast foundation is required to resist overturning.*

#### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	30 in			30 in			30 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	280 lbs	693 lbs	280 lbs	729 lbs	1960 lbs	729 lbs	82 lbs	203 lbs	82 lbs
$F_v$	3 lbs	0 lbs	3 lbs	8 lbs	0 lbs	8 lbs	1 lbs	0 lbs	1 lbs
$P_{total}$	7685 lbs	5981 lbs	7685 lbs	7778 lbs	5981 lbs	7778 lbs	2247 lbs	5981 lbs	2247 lbs
$M$	9 lbs-ft	0 lbs-ft	9 lbs-ft	26 lbs-ft	0 lbs-ft	26 lbs-ft	3 lbs-ft	0 lbs-ft	3 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft	0.42 ft
$f_{min}$	278.6 psf	217.5 psf	278.6 psf	280.5 psf	217.5 psf	280.5 psf	81.4 psf	217.5 psf	81.4 psf
$f_{max}$	280.2 psf	217.5 psf	280.2 psf	285.1 psf	217.5 psf	285.1 psf	82.0 psf	217.5 psf	82.0 psf



Maximum Bearing Pressure = 285 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 30in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 132in long x 30in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.513 k
Allowable Uplift =	1.214 k
Utilization =	<u>42%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.053 k
Allowable Uplift =	4.357 k
Utilization =	<u>47%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.409 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>32%</u>

#### Rear Strut

Maximum Axial Load =	3.931 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>53%</u>

#### Diagonal Strut

Maximum Axial Load =	2.656 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>36%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 135 \text{ in}$$

$$J = 0.432$$

$$373.473$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 135$$

$$J = 0.432$$

$$237.507$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$
  

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$
  

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-42.559	-42.559	0	0
2	M14	y	-42.559	-42.559	0	0
3	M15	y	-70.932	-70.932	0	0
4	M16	y	-70.932	-70.932	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	95.759	95.759	0	0
2	M14	y	74.479	74.479	0	0
3	M15	y	42.559	42.559	0	0
4	M16	y	42.559	42.559	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.8W	Yes Y		1 1.2	3 1.6	4 .8													
2	LRFD 1.2D + 1.6W + 0.5S	Yes Y		1 1.2	3 .5	4 1.6													
3	LRFD 0.9D + 1.6W	Yes Y		2 .9				5 1.6											
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1 1.54	3 .2			6 1.3											
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1 .56				6 1.3											
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1 1.54	3 .2			6 1.25											
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1 .56				6 1.25											



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 35° 90mph 30psf 11.25ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	140.283	1	188.989	2	-1.104	12	.012	2	-.011	15	1.017	3
28			min	7.802	15	-302.348	3	-30.555	1	-.002	3	-.197	1	-.602	2
29		15	max	140.283	1	74.863	2	15.759	1	.012	2	-.011	12	1.279	3
30			min	7.802	15	-116.317	3	.891	15	-.002	3	-.206	1	-.767	2
31		16	max	140.283	1	69.714	3	62.072	1	.012	2	-.008	12	1.308	3
32			min	7.802	15	-39.262	2	3.453	15	-.002	3	-.157	1	-.789	2
33		17	max	140.283	1	255.745	3	108.386	1	.012	2	-.001	12	1.104	3
34			min	7.802	15	-153.388	2	6.015	15	-.002	3	-.051	1	-.669	2
35		18	max	140.283	1	441.776	3	154.699	1	.012	2	.114	1	.668	3
36			min	7.802	15	-267.513	2	8.576	15	-.002	3	.006	15	-.406	2
37		19	max	140.283	1	627.807	3	201.013	1	.012	2	.336	1	0	2
38			min	7.802	15	-381.638	2	11.138	15	-.002	3	.019	15	0	3
39	M14	1	max	60.573	1	400.364	2	-11.451	15	.007	3	.378	1	0	1
40			min	3.375	15	-495.442	3	-206.668	1	-.009	2	.021	15	0	3
41		2	max	60.573	1	286.239	2	-8.889	15	.007	3	.149	1	.53	3
42			min	3.375	15	-351.971	3	-160.354	1	-.009	2	.008	15	-.429	2
43		3	max	60.573	1	172.114	2	-6.327	15	.007	3	0	3	.88	3
44			min	3.375	15	-208.5	3	-114.041	1	-.009	2	-.023	1	-.716	2
45		4	max	60.573	1	57.988	2	-3.766	15	.007	3	-.006	12	1.051	3
46			min	3.375	15	-65.029	3	-67.727	1	-.009	2	-.136	1	-.859	2
47		5	max	60.573	1	78.442	3	-1.204	15	.007	3	-.01	12	1.042	3
48			min	3.375	15	-56.137	2	-21.413	1	-.009	2	-.192	1	-.861	2
49		6	max	60.573	1	221.913	3	24.9	1	.007	3	-.011	15	.855	3
50			min	3.375	15	-170.262	2	.806	12	-.009	2	-.19	1	-.719	2
51		7	max	60.573	1	365.384	3	71.214	1	.007	3	-.007	15	.488	3
52			min	3.375	15	-284.388	2	3.367	12	-.009	2	-.13	1	-.435	2
53		8	max	60.573	1	508.855	3	117.527	1	.007	3	0	10	.006	9
54			min	3.375	15	-398.513	2	5.929	12	-.009	2	-.012	1	-.059	3
55		9	max	60.573	1	652.326	3	163.841	1	.007	3	.164	1	.561	2
56			min	3.375	15	-512.639	2	8.49	12	-.009	2	.007	12	-.784	3
57		10	max	60.573	1	795.796	3	210.154	1	.009	2	.398	1	1.273	2
58			min	3.375	15	-626.764	2	-118.108	14	-.007	3	.019	12	-1.69	3
59		11	max	60.573	1	512.639	2	-8.49	12	.009	2	.164	1	.561	2
60			min	3.375	15	-652.326	3	-163.841	1	-.007	3	.007	12	-.784	3
61		12	max	60.573	1	398.513	2	-5.929	12	.009	2	0	10	.006	9
62			min	3.375	15	-508.855	3	-117.527	1	-.007	3	-.012	1	-.059	3
63		13	max	60.573	1	284.388	2	-3.367	12	.009	2	-.007	15	.488	3
64			min	3.375	15	-365.384	3	-71.214	1	-.007	3	-.13	1	-.435	2
65		14	max	60.573	1	170.262	2	-.806	12	.009	2	-.011	15	.855	3
66			min	3.375	15	-221.913	3	-24.9	1	-.007	3	-.19	1	-.719	2
67		15	max	60.573	1	56.137	2	21.413	1	.009	2	-.01	12	1.042	3
68			min	3.375	15	-78.442	3	1.204	15	-.007	3	-.192	1	-.861	2
69		16	max	60.573	1	65.029	3	67.727	1	.009	2	-.006	12	1.051	3
70			min	3.375	15	-57.988	2	3.766	15	-.007	3	-.136	1	-.859	2
71		17	max	60.573	1	208.5	3	114.041	1	.009	2	0	3	.88	3
72			min	3.375	15	-172.114	2	6.327	15	-.007	3	-.023	1	-.716	2
73		18	max	60.573	1	351.971	3	160.354	1	.009	2	.149	1	.53	3
74			min	3.375	15	-286.239	2	8.889	15	-.007	3	.008	15	-.429	2
75		19	max	60.573	1	495.442	3	206.668	1	.009	2	.378	1	0	1
76			min	3.375	15	-400.364	2	11.451	15	-.007	3	.021	15	0	3
77	M15	1	max	-3.562	15	591.894	2	-11.448	15	.009	2	.378	1	0	2
78			min	-63.907	1	-280.069	3	-206.635	1	-.007	3	.021	15	0	3
79		2	max	-3.562	15	421.023	2	-8.886	15	.009	2	.149	1	.3	3
80			min	-63.907	1	-200.438	3	-160.321	1	-.007	3	.008	15	-.633	2
81		3	max	-3.562	15	250.151	2	-6.324	15	.009	2	0	3	.501	3
82			min	-63.907	1	-120.807	3	-114.008	1	-.007	3	-.023	1	-1.053	2
83		4	max	-3.562	15	79.28	2	-3.763	15	.009	2	-.007	12	.602	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-63.907	1	-41.176	3	-67.694	1	-.007	3	-.136	1	-1.258	2
85		5	max	-3.562	15	38.455	3	-1.201	15	.009	2	-.01	12	.604	3
86			min	-63.907	1	-91.592	2	-21.381	1	-.007	3	-.192	1	-1.251	2
87		6	max	-3.562	15	118.086	3	24.933	1	.009	2	-.011	15	.506	3
88			min	-63.907	1	-262.463	2	.853	12	-.007	3	-.19	1	-1.029	2
89		7	max	-3.562	15	197.717	3	71.247	1	.009	2	-.007	15	.309	3
90			min	-63.907	1	-433.334	2	3.414	12	-.007	3	-.13	1	-.595	2
91		8	max	-3.562	15	277.348	3	117.56	1	.009	2	0	10	.054	2
92			min	-63.907	1	-604.206	2	5.976	12	-.007	3	-.012	1	0	15
93		9	max	-3.562	15	356.979	3	163.874	1	.009	2	.164	1	.916	2
94			min	-63.907	1	-775.077	2	8.537	12	-.007	3	.007	12	-.385	3
95		10	max	-3.562	15	945.948	2	112.71	11	.007	3	.398	1	1.992	2
96			min	-63.907	1	-563.726	11	-210.187	1	-.009	2	.019	12	-.881	3
97		11	max	-3.562	15	775.077	2	-8.537	12	.007	3	.164	1	.916	2
98			min	-63.907	1	-356.979	3	-163.874	1	-.009	2	.007	12	-.385	3
99		12	max	-3.562	15	604.206	2	-5.976	12	.007	3	0	10	.054	2
100			min	-63.907	1	-277.348	3	-117.56	1	-.009	2	-.012	1	0	15
101		13	max	-3.562	15	433.334	2	-3.414	12	.007	3	-.007	15	.309	3
102			min	-63.907	1	-197.717	3	-71.247	1	-.009	2	-.13	1	-.595	2
103		14	max	-3.562	15	262.463	2	-.853	12	.007	3	-.011	15	.506	3
104			min	-63.907	1	-118.086	3	-24.933	1	-.009	2	-.19	1	-1.029	2
105		15	max	-3.562	15	91.592	2	21.381	1	.007	3	-.01	12	.604	3
106			min	-63.907	1	-38.455	3	1.201	15	-.009	2	-.192	1	-1.251	2
107		16	max	-3.562	15	41.176	3	67.694	1	.007	3	-.007	12	.602	3
108			min	-63.907	1	-79.28	2	3.763	15	-.009	2	-.136	1	-1.258	2
109		17	max	-3.562	15	120.807	3	114.008	1	.007	3	0	3	.501	3
110			min	-63.907	1	-250.151	2	6.324	15	-.009	2	-.023	1	-1.053	2
111		18	max	-3.562	15	200.438	3	160.321	1	.007	3	.149	1	.3	3
112			min	-63.907	1	-421.023	2	8.886	15	-.009	2	.008	15	-.633	2
113		19	max	-3.562	15	280.069	3	206.635	1	.007	3	.378	1	0	2
114			min	-63.907	1	-591.894	2	11.448	15	-.009	2	.021	15	0	3
115	M16	1	max	-8.427	15	573.826	2	-11.147	15	.009	2	.338	1	0	2
116			min	-151.294	1	-265.675	3	-201.252	1	-.01	3	.019	15	0	3
117		2	max	-8.427	15	402.955	2	-8.586	15	.009	2	.115	1	.282	3
118			min	-151.294	1	-186.044	3	-154.939	1	-.01	3	.006	15	-.61	2
119		3	max	-8.427	15	232.083	2	-6.024	15	.009	2	-.002	12	.465	3
120			min	-151.294	1	-106.413	3	-108.625	1	-.01	3	-.05	1	-1.007	2
121		4	max	-8.427	15	61.212	2	-3.462	15	.009	2	-.008	12	.548	3
122			min	-151.294	1	-26.782	3	-62.312	1	-.01	3	-.157	1	-1.191	2
123		5	max	-8.427	15	52.849	3	-.901	15	.009	2	-.011	12	.532	3
124			min	-151.294	1	-109.659	2	-15.998	1	-.01	3	-.205	1	-1.16	2
125		6	max	-8.427	15	132.48	3	30.315	1	.009	2	-.011	15	.416	3
126			min	-151.294	1	-280.531	2	1.255	12	-.01	3	-.197	1	-.917	2
127		7	max	-8.427	15	212.111	3	76.629	1	.009	2	-.007	15	.201	3
128			min	-151.294	1	-451.402	2	3.816	12	-.01	3	-.13	1	-.459	2
129		8	max	-8.427	15	291.742	3	122.942	1	.009	2	0	10	.212	2
130			min	-151.294	1	-622.273	2	6.377	12	-.01	3	-.005	1	-.114	3
131		9	max	-8.427	15	371.373	3	169.256	1	.009	2	.178	1	1.097	2
132			min	-151.294	1	-793.145	2	8.939	12	-.01	3	.008	12	-.528	3
133		10	max	-8.427	15	451.004	3	215.57	1	.01	3	.418	1	2.195	2
134			min	-151.294	1	-964.016	2	-121.197	14	-.009	2	.021	12	-1.042	3
135		11	max	-8.427	15	793.145	2	-8.939	12	.01	3	.178	1	1.097	2
136			min	-151.294	1	-371.373	3	-169.256	1	-.009	2	.008	12	-.528	3
137		12	max	-8.427	15	622.273	2	-6.377	12	.01	3	0	10	.212	2
138			min	-151.294	1	-291.742	3	-122.942	1	-.009	2	-.005	1	-.114	3
139		13	max	-8.427	15	451.402	2	-3.816	12	.01	3	-.007	15	.201	3
140			min	-151.294	1	-212.111	3	-76.629	1	-.009	2	-.13	1	-.459	2





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-8.427	15	280.531	2	-1.255	12	.01	3	-.011	15	.416	3
142			min	-151.294	1	-132.48	3	-30.315	1	-.009	2	-.197	1	-.917	2
143		15	max	-8.427	15	109.659	2	15.998	1	.01	3	-.011	12	.532	3
144			min	-151.294	1	-52.849	3	.901	15	-.009	2	-.205	1	-1.16	2
145		16	max	-8.427	15	26.782	3	62.312	1	.01	3	-.008	12	.548	3
146			min	-151.294	1	-61.212	2	3.462	15	-.009	2	-.157	1	-1.191	2
147		17	max	-8.427	15	106.413	3	108.625	1	.01	3	-.002	12	.465	3
148			min	-151.294	1	-232.083	2	6.024	15	-.009	2	-.05	1	-1.007	2
149		18	max	-8.427	15	186.044	3	154.939	1	.01	3	.115	1	.282	3
150			min	-151.294	1	-402.955	2	8.586	15	-.009	2	.006	15	-.61	2
151		19	max	-8.427	15	265.675	3	201.252	1	.01	3	.338	1	0	2
152			min	-151.294	1	-573.826	2	11.147	15	-.009	2	.019	15	0	3
153	M2	1	max	924.943	2	2.016	4	.475	1	0	12	0	3	0	1
154			min	-1195.846	3	.474	15	.026	15	0	1	0	2	0	1
155		2	max	925.463	2	1.898	4	.475	1	0	12	0	1	0	15
156			min	-1195.455	3	.446	15	.026	15	0	1	0	15	0	4
157		3	max	925.984	2	1.779	4	.475	1	0	12	0	1	0	15
158			min	-1195.065	3	.419	15	.026	15	0	1	0	15	-.001	4
159		4	max	926.505	2	1.66	4	.475	1	0	12	0	1	0	15
160			min	-1194.674	3	.391	15	.026	15	0	1	0	15	-.002	4
161		5	max	927.026	2	1.541	4	.475	1	0	12	0	1	0	15
162			min	-1194.284	3	.363	15	.026	15	0	1	0	15	-.003	4
163		6	max	927.546	2	1.422	4	.475	1	0	12	0	1	0	15
164			min	-1193.893	3	.335	15	.026	15	0	1	0	15	-.003	4
165		7	max	928.067	2	1.303	4	.475	1	0	12	0	1	0	15
166			min	-1193.503	3	.307	15	.026	15	0	1	0	15	-.004	4
167		8	max	928.588	2	1.184	4	.475	1	0	12	.001	1	0	15
168			min	-1193.112	3	.279	15	.026	15	0	1	0	15	-.004	4
169		9	max	929.108	2	1.066	4	.475	1	0	12	.001	1	-.001	15
170			min	-1192.722	3	.251	15	.026	15	0	1	0	15	-.004	4
171		10	max	929.629	2	.947	4	.475	1	0	12	.002	1	-.001	15
172			min	-1192.331	3	.218	12	.026	15	0	1	0	15	-.005	4
173		11	max	930.15	2	.828	4	.475	1	0	12	.002	1	-.001	15
174			min	-1191.941	3	.171	12	.026	15	0	1	0	15	-.005	4
175		12	max	930.67	2	.728	2	.475	1	0	12	.002	1	-.001	15
176			min	-1191.55	3	.125	12	.026	15	0	1	0	15	-.005	4
177		13	max	931.191	2	.636	2	.475	1	0	12	.002	1	-.001	15
178			min	-1191.16	3	.079	12	.026	15	0	1	0	15	-.006	4
179		14	max	931.712	2	.543	2	.475	1	0	12	.002	1	-.001	15
180			min	-1190.769	3	.032	12	.026	15	0	1	0	15	-.006	4
181		15	max	932.232	2	.45	2	.475	1	0	12	.002	1	-.001	15
182			min	-1190.379	3	-.036	3	.026	15	0	1	0	15	-.006	4
183		16	max	932.753	2	.358	2	.475	1	0	12	.003	1	-.001	15
184			min	-1189.988	3	-.105	3	.026	15	0	1	0	15	-.006	4
185		17	max	933.274	2	.265	2	.475	1	0	12	.003	1	-.001	15
186			min	-1189.598	3	-.175	3	.026	15	0	1	0	15	-.006	4
187		18	max	933.795	2	.173	2	.475	1	0	12	.003	1	-.001	15
188			min	-1189.207	3	-.244	3	.026	15	0	1	0	15	-.006	4
189		19	max	934.315	2	.08	2	.475	1	0	12	.003	1	-.001	12
190			min	-1188.817	3	-.314	3	.026	15	0	1	0	15	-.006	4
191	M3	1	max	690.978	2	7.66	4	.401	1	0	12	0	1	.006	4
192			min	-838.18	3	1.801	15	.022	15	0	1	0	15	.001	12
193		2	max	690.808	2	6.899	4	.401	1	0	12	0	1	.003	2
194			min	-838.307	3	1.622	15	.022	15	0	1	0	15	0	12
195		3	max	690.638	2	6.138	4	.401	1	0	12	0	1	.001	2
196			min	-838.435	3	1.443	15	.022	15	0	1	0	15	-.001	3
197		4	max	690.467	2	5.377	4	.401	1	0	12	.001	1	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-838.563	3	1.264	15	.022	15	0	1	0	15	-.002	3
199		5	max	690.297	2	4.616	4	.401	1	0	12	.001	1	0	15
200			min	-838.691	3	1.085	15	.022	15	0	1	0	15	-.004	4
201		6	max	690.127	2	3.855	4	.401	1	0	12	.001	1	-.001	15
202			min	-838.818	3	.907	15	.022	15	0	1	0	15	-.006	4
203		7	max	689.956	2	3.094	4	.401	1	0	12	.002	1	-.002	15
204			min	-838.946	3	.728	15	.022	15	0	1	0	15	-.007	4
205		8	max	689.786	2	2.334	4	.401	1	0	12	.002	1	-.002	15
206			min	-839.074	3	.549	15	.022	15	0	1	0	15	-.008	4
207		9	max	689.616	2	1.573	4	.401	1	0	12	.002	1	-.002	15
208			min	-839.202	3	.37	15	.022	15	0	1	0	15	-.009	4
209		10	max	689.445	2	.812	4	.401	1	0	12	.002	1	-.002	15
210			min	-839.329	3	.187	12	.022	15	0	1	0	15	-.01	4
211		11	max	689.275	2	.19	2	.401	1	0	12	.002	1	-.002	15
212			min	-839.457	3	-.178	3	.022	15	0	1	0	15	-.01	4
213		12	max	689.105	2	-.167	15	.401	1	0	12	.002	1	-.002	15
214			min	-839.585	3	-.71	4	.022	15	0	1	0	15	-.01	4
215		13	max	688.934	2	-.346	15	.401	1	0	12	.003	1	-.002	15
216			min	-839.713	3	-1.471	4	.022	15	0	1	0	15	-.009	4
217		14	max	688.764	2	-.524	15	.401	1	0	12	.003	1	-.002	15
218			min	-839.84	3	-2.232	4	.022	15	0	1	0	15	-.009	4
219		15	max	688.594	2	-.703	15	.401	1	0	12	.003	1	-.002	15
220			min	-839.968	3	-2.993	4	.022	15	0	1	0	15	-.008	4
221		16	max	688.423	2	-.882	15	.401	1	0	12	.003	1	-.001	15
222			min	-840.096	3	-3.754	4	.022	15	0	1	0	15	-.006	4
223		17	max	688.253	2	-1.061	15	.401	1	0	12	.003	1	-.001	15
224			min	-840.224	3	-4.515	4	.022	15	0	1	0	15	-.004	4
225		18	max	688.083	2	-1.24	15	.401	1	0	12	.003	1	0	15
226			min	-840.351	3	-5.276	4	.022	15	0	1	0	15	-.002	4
227		19	max	687.912	2	-1.419	15	.401	1	0	12	.004	1	0	1
228			min	-840.479	3	-6.037	4	.022	15	0	1	0	15	0	1
229	M4	1	max	988.667	1	0	1	-.862	15	0	1	.003	1	0	1
230			min	49.901	15	0	1	-15.526	1	0	1	0	15	0	1
231		2	max	988.837	1	0	1	-.862	15	0	1	.002	1	0	1
232			min	49.952	15	0	1	-15.526	1	0	1	0	15	0	1
233		3	max	989.007	1	0	1	-.862	15	0	1	0	12	0	1
234			min	50.003	15	0	1	-15.526	1	0	1	0	1	0	1
235		4	max	989.178	1	0	1	-.862	15	0	1	0	15	0	1
236			min	50.055	15	0	1	-15.526	1	0	1	-.002	1	0	1
237		5	max	989.348	1	0	1	-.862	15	0	1	0	15	0	1
238			min	50.106	15	0	1	-15.526	1	0	1	-.004	1	0	1
239		6	max	989.518	1	0	1	-.862	15	0	1	0	15	0	1
240			min	50.158	15	0	1	-15.526	1	0	1	-.006	1	0	1
241		7	max	989.689	1	0	1	-.862	15	0	1	0	15	0	1
242			min	50.209	15	0	1	-15.526	1	0	1	-.007	1	0	1
243		8	max	989.859	1	0	1	-.862	15	0	1	0	15	0	1
244			min	50.26	15	0	1	-15.526	1	0	1	-.009	1	0	1
245		9	max	990.029	1	0	1	-.862	15	0	1	0	15	0	1
246			min	50.312	15	0	1	-15.526	1	0	1	-.011	1	0	1
247		10	max	990.2	1	0	1	-.862	15	0	1	0	15	0	1
248			min	50.363	15	0	1	-15.526	1	0	1	-.013	1	0	1
249		11	max	990.37	1	0	1	-.862	15	0	1	0	15	0	1
250			min	50.415	15	0	1	-15.526	1	0	1	-.014	1	0	1
251		12	max	990.54	1	0	1	-.862	15	0	1	0	15	0	1
252			min	50.466	15	0	1	-15.526	1	0	1	-.016	1	0	1
253		13	max	990.711	1	0	1	-.862	15	0	1	-.001	15	0	1
254			min	50.517	15	0	1	-15.526	1	0	1	-.018	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	990.881	1	0	1	-.862	15	0	1	-.001	15	0	1
256		min	50.569	15	0	1	-15.526	1	0	1	-.02	1	0	1
257	15	max	991.052	1	0	1	-.862	15	0	1	-.001	15	0	1
258		min	50.62	15	0	1	-15.526	1	0	1	-.022	1	0	1
259	16	max	991.222	1	0	1	-.862	15	0	1	-.001	15	0	1
260		min	50.672	15	0	1	-15.526	1	0	1	-.023	1	0	1
261	17	max	991.392	1	0	1	-.862	15	0	1	-.001	15	0	1
262		min	50.723	15	0	1	-15.526	1	0	1	-.025	1	0	1
263	18	max	991.563	1	0	1	-.862	15	0	1	-.001	15	0	1
264		min	50.774	15	0	1	-15.526	1	0	1	-.027	1	0	1
265	19	max	991.733	1	0	1	-.862	15	0	1	-.002	15	0	1
266		min	50.826	15	0	1	-15.526	1	0	1	-.029	1	0	1
267	M6	1	max	2992.998	2	2.201	2	0	1	0	0	1	0	1
268		min	-3931.078	3	.297	12	0	1	0	1	0	1	0	1
269	2	max	2993.519	2	2.108	2	0	1	0	1	0	1	0	12
270		min	-3930.687	3	.25	12	0	1	0	1	0	1	0	2
271	3	max	2994.04	2	2.016	2	0	1	0	1	0	1	0	12
272		min	-3930.297	3	.204	12	0	1	0	1	0	1	-.002	2
273	4	max	2994.56	2	1.923	2	0	1	0	1	0	1	0	12
274		min	-3929.906	3	.158	12	0	1	0	1	0	1	-.002	2
275	5	max	2995.081	2	1.83	2	0	1	0	1	0	1	0	12
276		min	-3929.516	3	.111	12	0	1	0	1	0	1	-.003	2
277	6	max	2995.602	2	1.738	2	0	1	0	1	0	1	0	12
278		min	-3929.125	3	.048	3	0	1	0	1	0	1	-.004	2
279	7	max	2996.123	2	1.645	2	0	1	0	1	0	1	0	12
280		min	-3928.735	3	-.021	3	0	1	0	1	0	1	-.004	2
281	8	max	2996.643	2	1.552	2	0	1	0	1	0	1	0	12
282		min	-3928.344	3	-.091	3	0	1	0	1	0	1	-.005	2
283	9	max	2997.164	2	1.46	2	0	1	0	1	0	1	0	12
284		min	-3927.953	3	-.16	3	0	1	0	1	0	1	-.005	2
285	10	max	2997.685	2	1.367	2	0	1	0	1	0	1	0	3
286		min	-3927.563	3	-.23	3	0	1	0	1	0	1	-.006	2
287	11	max	2998.205	2	1.275	2	0	1	0	1	0	1	0	3
288		min	-3927.172	3	-.299	3	0	1	0	1	0	1	-.006	2
289	12	max	2998.726	2	1.182	2	0	1	0	1	0	1	0	3
290		min	-3926.782	3	-.369	3	0	1	0	1	0	1	-.007	2
291	13	max	2999.247	2	1.089	2	0	1	0	1	0	1	0	3
292		min	-3926.391	3	-.438	3	0	1	0	1	0	1	-.007	2
293	14	max	2999.767	2	.997	2	0	1	0	1	0	1	0	3
294		min	-3926.001	3	-.508	3	0	1	0	1	0	1	-.007	2
295	15	max	3000.288	2	.904	2	0	1	0	1	0	1	0	3
296		min	-3925.61	3	-.577	3	0	1	0	1	0	1	-.008	2
297	16	max	3000.809	2	.812	2	0	1	0	1	0	1	0	3
298		min	-3925.22	3	-.647	3	0	1	0	1	0	1	-.008	2
299	17	max	3001.329	2	.719	2	0	1	0	1	0	1	0	3
300		min	-3924.829	3	-.716	3	0	1	0	1	0	1	-.008	2
301	18	max	3001.85	2	.626	2	0	1	0	1	0	1	.001	3
302		min	-3924.439	3	-.785	3	0	1	0	1	0	1	-.009	2
303	19	max	3002.371	2	.534	2	0	1	0	1	0	1	.001	3
304		min	-3924.048	3	-.855	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	2615.338	2	7.694	4	0	1	0	1	0	.009	2
306		min	-2653.86	3	1.806	15	0	1	0	1	0	1	-.001	3
307	2	max	2615.167	2	6.933	4	0	1	0	1	0	1	.006	2
308		min	-2653.988	3	1.628	15	0	1	0	1	0	1	-.003	3
309	3	max	2614.997	2	6.172	4	0	1	0	1	0	1	.004	2
310		min	-2654.116	3	1.449	15	0	1	0	1	0	1	-.004	3
311	4	max	2614.826	2	5.411	4	0	1	0	1	0	1	.002	2



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2654.244	3	1.27	15	0	1	0	1	0	1	-.005	3
313	5	max	2614.656	2	4.65	4	0	1	0	1	0	1	0	2
314		min	-2654.371	3	1.091	15	0	1	0	1	0	1	-.006	3
315	6	max	2614.486	2	3.889	4	0	1	0	1	0	1	-.001	15
316		min	-2654.499	3	.912	15	0	1	0	1	0	1	-.007	3
317	7	max	2614.315	2	3.128	4	0	1	0	1	0	1	-.002	15
318		min	-2654.627	3	.733	15	0	1	0	1	0	1	-.008	3
319	8	max	2614.145	2	2.367	4	0	1	0	1	0	1	-.002	15
320		min	-2654.755	3	.49	12	0	1	0	1	0	1	-.008	4
321	9	max	2613.975	2	1.765	2	0	1	0	1	0	1	-.002	15
322		min	-2654.882	3	.193	12	0	1	0	1	0	1	-.009	4
323	10	max	2613.804	2	1.172	2	0	1	0	1	0	1	-.002	15
324		min	-2655.01	3	-.197	3	0	1	0	1	0	1	-.01	4
325	11	max	2613.634	2	.579	2	0	1	0	1	0	1	-.002	15
326		min	-2655.138	3	-.642	3	0	1	0	1	0	1	-.01	4
327	12	max	2613.464	2	-.014	2	0	1	0	1	0	1	-.002	15
328		min	-2655.266	3	-1.086	3	0	1	0	1	0	1	-.01	4
329	13	max	2613.293	2	-.34	15	0	1	0	1	0	1	-.002	15
330		min	-2655.393	3	-1.531	3	0	1	0	1	0	1	-.009	4
331	14	max	2613.123	2	-.519	15	0	1	0	1	0	1	-.002	15
332		min	-2655.521	3	-2.199	4	0	1	0	1	0	1	-.009	4
333	15	max	2612.953	2	-.698	15	0	1	0	1	0	1	-.002	15
334		min	-2655.649	3	-2.96	4	0	1	0	1	0	1	-.007	4
335	16	max	2612.782	2	-.877	15	0	1	0	1	0	1	-.001	15
336		min	-2655.777	3	-3.721	4	0	1	0	1	0	1	-.006	4
337	17	max	2612.612	2	-1.056	15	0	1	0	1	0	1	-.001	15
338		min	-2655.904	3	-4.482	4	0	1	0	1	0	1	-.004	4
339	18	max	2612.442	2	-1.234	15	0	1	0	1	0	1	0	15
340		min	-2656.032	3	-5.243	4	0	1	0	1	0	1	-.002	4
341	19	max	2612.271	2	-1.413	15	0	1	0	1	0	1	0	1
342		min	-2656.16	3	-6.004	4	0	1	0	1	0	1	0	1
343	M8	1	max	2405.633	1	0	1	0	1	0	1	0	1	1
344		min	105.271	15	0	1	0	1	0	1	0	1	0	1
345	2	max	2405.803	1	0	1	0	1	0	1	0	1	0	1
346		min	105.323	15	0	1	0	1	0	1	0	1	0	1
347	3	max	2405.974	1	0	1	0	1	0	1	0	1	0	1
348		min	105.374	15	0	1	0	1	0	1	0	1	0	1
349	4	max	2406.144	1	0	1	0	1	0	1	0	1	0	1
350		min	105.425	15	0	1	0	1	0	1	0	1	0	1
351	5	max	2406.314	1	0	1	0	1	0	1	0	1	0	1
352		min	105.477	15	0	1	0	1	0	1	0	1	0	1
353	6	max	2406.485	1	0	1	0	1	0	1	0	1	0	1
354		min	105.528	15	0	1	0	1	0	1	0	1	0	1
355	7	max	2406.655	1	0	1	0	1	0	1	0	1	0	1
356		min	105.58	15	0	1	0	1	0	1	0	1	0	1
357	8	max	2406.825	1	0	1	0	1	0	1	0	1	0	1
358		min	105.631	15	0	1	0	1	0	1	0	1	0	1
359	9	max	2406.996	1	0	1	0	1	0	1	0	1	0	1
360		min	105.682	15	0	1	0	1	0	1	0	1	0	1
361	10	max	2407.166	1	0	1	0	1	0	1	0	1	0	1
362		min	105.734	15	0	1	0	1	0	1	0	1	0	1
363	11	max	2407.336	1	0	1	0	1	0	1	0	1	0	1
364		min	105.785	15	0	1	0	1	0	1	0	1	0	1
365	12	max	2407.507	1	0	1	0	1	0	1	0	1	0	1
366		min	105.837	15	0	1	0	1	0	1	0	1	0	1
367	13	max	2407.677	1	0	1	0	1	0	1	0	1	0	1
368		min	105.888	15	0	1	0	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	2407.847	1	0	1	0	1	0	1	0	1	0	1
370			min	105.939	15	0	1	0	1	0	1	0	1	0	1
371		15	max	2408.018	1	0	1	0	1	0	1	0	1	0	1
372			min	105.991	15	0	1	0	1	0	1	0	1	0	1
373		16	max	2408.188	1	0	1	0	1	0	1	0	1	0	1
374			min	106.042	15	0	1	0	1	0	1	0	1	0	1
375		17	max	2408.359	1	0	1	0	1	0	1	0	1	0	1
376			min	106.093	15	0	1	0	1	0	1	0	1	0	1
377		18	max	2408.529	1	0	1	0	1	0	1	0	1	0	1
378			min	106.145	15	0	1	0	1	0	1	0	1	0	1
379		19	max	2408.699	1	0	1	0	1	0	1	0	1	0	1
380			min	106.196	15	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	924.943	2	2.016	4	-.026	15	0	1	0	2	0	1
382			min	-1195.846	3	.474	15	-.475	1	0	12	0	3	0	1
383		2	max	925.463	2	1.898	4	-.026	15	0	1	0	15	0	15
384			min	-1195.455	3	.446	15	-.475	1	0	12	0	1	0	4
385		3	max	925.984	2	1.779	4	-.026	15	0	1	0	15	0	15
386			min	-1195.065	3	.419	15	-.475	1	0	12	0	1	-.001	4
387		4	max	926.505	2	1.66	4	-.026	15	0	1	0	15	0	15
388			min	-1194.674	3	.391	15	-.475	1	0	12	0	1	-.002	4
389		5	max	927.026	2	1.541	4	-.026	15	0	1	0	15	0	15
390			min	-1194.284	3	.363	15	-.475	1	0	12	0	1	-.003	4
391		6	max	927.546	2	1.422	4	-.026	15	0	1	0	15	0	15
392			min	-1193.893	3	.335	15	-.475	1	0	12	0	1	-.003	4
393		7	max	928.067	2	1.303	4	-.026	15	0	1	0	15	0	15
394			min	-1193.503	3	.307	15	-.475	1	0	12	0	1	-.004	4
395		8	max	928.588	2	1.184	4	-.026	15	0	1	0	15	0	15
396			min	-1193.112	3	.279	15	-.475	1	0	12	-.001	1	-.004	4
397		9	max	929.108	2	1.066	4	-.026	15	0	1	0	15	-.001	15
398			min	-1192.722	3	.251	15	-.475	1	0	12	-.001	1	-.004	4
399		10	max	929.629	2	.947	4	-.026	15	0	1	0	15	-.001	15
400			min	-1192.331	3	.218	12	-.475	1	0	12	-.002	1	-.005	4
401		11	max	930.15	2	.828	4	-.026	15	0	1	0	15	-.001	15
402			min	-1191.941	3	.171	12	-.475	1	0	12	-.002	1	-.005	4
403		12	max	930.67	2	.728	2	-.026	15	0	1	0	15	-.001	15
404			min	-1191.55	3	.125	12	-.475	1	0	12	-.002	1	-.005	4
405		13	max	931.191	2	.636	2	-.026	15	0	1	0	15	-.001	15
406			min	-1191.16	3	.079	12	-.475	1	0	12	-.002	1	-.006	4
407		14	max	931.712	2	.543	2	-.026	15	0	1	0	15	-.001	15
408			min	-1190.769	3	.032	12	-.475	1	0	12	-.002	1	-.006	4
409		15	max	932.232	2	.45	2	-.026	15	0	1	0	15	-.001	15
410			min	-1190.379	3	-.036	3	-.475	1	0	12	-.002	1	-.006	4
411		16	max	932.753	2	.358	2	-.026	15	0	1	0	15	-.001	15
412			min	-1189.988	3	-.105	3	-.475	1	0	12	-.003	1	-.006	4
413		17	max	933.274	2	.265	2	-.026	15	0	1	0	15	-.001	15
414			min	-1189.598	3	-.175	3	-.475	1	0	12	-.003	1	-.006	4
415		18	max	933.795	2	.173	2	-.026	15	0	1	0	15	-.001	15
416			min	-1189.207	3	-.244	3	-.475	1	0	12	-.003	1	-.006	4
417		19	max	934.315	2	.08	2	-.026	15	0	1	0	15	-.001	12
418			min	-1188.817	3	-.314	3	-.475	1	0	12	-.003	1	-.006	4
419	M11	1	max	690.978	2	7.66	4	-.022	15	0	1	0	15	.006	4
420			min	-838.18	3	1.801	15	-.401	1	0	12	0	1	.001	12
421		2	max	690.808	2	6.899	4	-.022	15	0	1	0	15	.003	2
422			min	-838.307	3	1.622	15	-.401	1	0	12	0	1	0	12
423		3	max	690.638	2	6.138	4	-.022	15	0	1	0	15	.001	2
424			min	-838.435	3	1.443	15	-.401	1	0	12	0	1	-.001	3
425		4	max	690.467	2	5.377	4	-.022	15	0	1	0	15	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426		min	-838.563	3	1.264	15	-.401	1	0	12	-.001	1	-.002	3
427	5	max	690.297	2	4.616	4	-.022	15	0	1	0	15	0	15
428		min	-838.691	3	1.085	15	-.401	1	0	12	-.001	1	-.004	4
429	6	max	690.127	2	3.855	4	-.022	15	0	1	0	15	-.001	15
430		min	-838.818	3	.907	15	-.401	1	0	12	-.001	1	-.006	4
431	7	max	689.956	2	3.094	4	-.022	15	0	1	0	15	-.002	15
432		min	-838.946	3	.728	15	-.401	1	0	12	-.002	1	-.007	4
433	8	max	689.786	2	2.334	4	-.022	15	0	1	0	15	-.002	15
434		min	-839.074	3	.549	15	-.401	1	0	12	-.002	1	-.008	4
435	9	max	689.616	2	1.573	4	-.022	15	0	1	0	15	-.002	15
436		min	-839.202	3	.37	15	-.401	1	0	12	-.002	1	-.009	4
437	10	max	689.445	2	.812	4	-.022	15	0	1	0	15	-.002	15
438		min	-839.329	3	.187	12	-.401	1	0	12	-.002	1	-.01	4
439	11	max	689.275	2	.19	2	-.022	15	0	1	0	15	-.002	15
440		min	-839.457	3	-.178	3	-.401	1	0	12	-.002	1	-.01	4
441	12	max	689.105	2	-.167	15	-.022	15	0	1	0	15	-.002	15
442		min	-839.585	3	-.71	4	-.401	1	0	12	-.002	1	-.01	4
443	13	max	688.934	2	-.346	15	-.022	15	0	1	0	15	-.002	15
444		min	-839.713	3	-1.471	4	-.401	1	0	12	-.003	1	-.009	4
445	14	max	688.764	2	-.524	15	-.022	15	0	1	0	15	-.002	15
446		min	-839.84	3	-2.232	4	-.401	1	0	12	-.003	1	-.009	4
447	15	max	688.594	2	-.703	15	-.022	15	0	1	0	15	-.002	15
448		min	-839.968	3	-2.993	4	-.401	1	0	12	-.003	1	-.008	4
449	16	max	688.423	2	-.882	15	-.022	15	0	1	0	15	-.001	15
450		min	-840.096	3	-3.754	4	-.401	1	0	12	-.003	1	-.006	4
451	17	max	688.253	2	-1.061	15	-.022	15	0	1	0	15	-.001	15
452		min	-840.224	3	-4.515	4	-.401	1	0	12	-.003	1	-.004	4
453	18	max	688.083	2	-1.24	15	-.022	15	0	1	0	15	0	15
454		min	-840.351	3	-5.276	4	-.401	1	0	12	-.003	1	-.002	4
455	19	max	687.912	2	-1.419	15	-.022	15	0	1	0	15	0	1
456		min	-840.479	3	-6.037	4	-.401	1	0	12	-.004	1	0	1
457	M12	1	max	988.667	1	0	15.526	1	0	1	0	15	0	1
458		min	49.901	15	0	1	.862	15	0	1	-.003	1	0	1
459	2	max	988.837	1	0	1	15.526	1	0	1	0	15	0	1
460		min	49.952	15	0	1	.862	15	0	1	-.002	1	0	1
461	3	max	989.007	1	0	1	15.526	1	0	1	0	1	0	1
462		min	50.003	15	0	1	.862	15	0	1	0	12	0	1
463	4	max	989.178	1	0	1	15.526	1	0	1	.002	1	0	1
464		min	50.055	15	0	1	.862	15	0	1	0	15	0	1
465	5	max	989.348	1	0	1	15.526	1	0	1	.004	1	0	1
466		min	50.106	15	0	1	.862	15	0	1	0	15	0	1
467	6	max	989.518	1	0	1	15.526	1	0	1	.006	1	0	1
468		min	50.158	15	0	1	.862	15	0	1	0	15	0	1
469	7	max	989.689	1	0	1	15.526	1	0	1	.007	1	0	1
470		min	50.209	15	0	1	.862	15	0	1	0	15	0	1
471	8	max	989.859	1	0	1	15.526	1	0	1	.009	1	0	1
472		min	50.26	15	0	1	.862	15	0	1	0	15	0	1
473	9	max	990.029	1	0	1	15.526	1	0	1	.011	1	0	1
474		min	50.312	15	0	1	.862	15	0	1	0	15	0	1
475	10	max	990.2	1	0	1	15.526	1	0	1	.013	1	0	1
476		min	50.363	15	0	1	.862	15	0	1	0	15	0	1
477	11	max	990.37	1	0	1	15.526	1	0	1	.014	1	0	1
478		min	50.415	15	0	1	.862	15	0	1	0	15	0	1
479	12	max	990.54	1	0	1	15.526	1	0	1	.016	1	0	1
480		min	50.466	15	0	1	.862	15	0	1	0	15	0	1
481	13	max	990.711	1	0	1	15.526	1	0	1	.018	1	0	1
482		min	50.517	15	0	1	.862	15	0	1	.001	15	0	1



Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	990.881	1	0	1	15.526	1	0	1	.02	1	0	1
484			min	50.569	15	0	1	.862	15	0	1	.001	15	0	1
485		15	max	991.052	1	0	1	15.526	1	0	1	.022	1	0	1
486			min	50.62	15	0	1	.862	15	0	1	.001	15	0	1
487		16	max	991.222	1	0	1	15.526	1	0	1	.023	1	0	1
488			min	50.672	15	0	1	.862	15	0	1	.001	15	0	1
489		17	max	991.392	1	0	1	15.526	1	0	1	.025	1	0	1
490			min	50.723	15	0	1	.862	15	0	1	.001	15	0	1
491		18	max	991.563	1	0	1	15.526	1	0	1	.027	1	0	1
492			min	50.774	15	0	1	.862	15	0	1	.001	15	0	1
493		19	max	991.733	1	0	1	15.526	1	0	1	.029	1	0	1
494			min	50.826	15	0	1	.862	15	0	1	.002	15	0	1
495	M1	1	max	201.021	1	627.767	3	-7.801	15	0	2	.336	1	.002	3
496			min	11.138	15	-380.97	2	-140.085	1	0	3	.019	15	-.012	2
497		2	max	201.842	1	626.887	3	-7.801	15	0	2	.262	1	.189	2
498			min	11.386	15	-382.144	2	-140.085	1	0	3	.015	15	-.329	3
499		3	max	523.254	3	452.912	2	-7.776	15	0	3	.188	1	.381	2
500			min	-298.885	2	-461.121	3	-139.878	1	0	2	.01	15	-.647	3
501		4	max	523.87	3	451.739	2	-7.776	15	0	3	.114	1	.148	1
502			min	-298.064	2	-462.001	3	-139.878	1	0	2	.006	15	-.403	3
503		5	max	524.486	3	450.566	2	-7.776	15	0	3	.041	1	-.003	15
504			min	-297.242	2	-462.881	3	-139.878	1	0	2	.002	15	-.159	3
505		6	max	525.102	3	449.392	2	-7.776	15	0	3	-.002	15	.085	3
506			min	-296.42	2	-463.762	3	-139.878	1	0	2	-.033	1	-.333	2
507		7	max	525.718	3	448.219	2	-7.776	15	0	3	-.006	15	.33	3
508			min	-295.599	2	-464.642	3	-139.878	1	0	2	-.107	1	-.57	2
509		8	max	526.335	3	447.045	2	-7.776	15	0	3	-.01	15	.576	3
510			min	-294.777	2	-465.522	3	-139.878	1	0	2	-.181	1	-.806	2
511		9	max	545.006	3	48.231	2	-11.151	15	0	9	.104	1	.671	3
512			min	-203.867	2	.359	15	-200.499	1	0	3	.006	15	-.924	2
513		10	max	545.622	3	47.058	2	-11.151	15	0	9	0	15	.654	3
514			min	-203.045	2	.005	15	-200.499	1	0	3	-.001	1	-.949	2
515		11	max	546.238	3	45.885	2	-11.151	15	0	9	-.006	15	.638	3
516			min	-202.224	2	-1.413	4	-200.499	1	0	3	-.107	1	-.974	2
517		12	max	564.836	3	312.736	3	-7.589	15	0	2	.179	1	.556	3
518			min	-117.415	10	-546.334	2	-136.68	1	0	3	.01	15	-.864	2
519		13	max	565.453	3	311.856	3	-7.589	15	0	2	.106	1	.391	3
520			min	-116.73	10	-547.507	2	-136.68	1	0	3	.006	15	-.575	2
521		14	max	566.069	3	310.976	3	-7.589	15	0	2	.034	1	.227	3
522			min	-116.046	10	-548.681	2	-136.68	1	0	3	.002	15	-.286	2
523		15	max	566.685	3	310.096	3	-7.589	15	0	2	-.002	15	.063	3
524			min	-115.361	10	-549.854	2	-136.68	1	0	3	-.038	1	-.019	9
525		16	max	567.301	3	309.216	3	-7.589	15	0	2	-.006	15	.294	2
526			min	-114.676	10	-551.027	2	-136.68	1	0	3	-.11	1	-.1	3
527		17	max	567.917	3	308.336	3	-7.589	15	0	2	-.01	15	.585	2
528			min	-113.992	10	-552.201	2	-136.68	1	0	3	-.182	1	-.263	3
529		18	max	-11.395	15	575.561	2	-8.428	15	0	3	-.014	15	.295	2
530			min	-202.068	1	-264.882	3	-151.486	1	0	2	-.258	1	-.13	3
531		19	max	-11.147	15	574.387	2	-8.428	15	0	3	-.019	15	.01	3
532			min	-201.246	1	-265.762	3	-151.486	1	0	2	-.338	1	-.009	2
533	M5	1	max	431.603	1	2092.757	3	0	1	0	1	0	1	.024	2
534			min	22.7	12	-1286.83	2	0	1	0	1	0	1	-.003	3
535		2	max	432.424	1	2091.877	3	0	1	0	1	0	1	.703	2
536			min	23.111	12	-1288.004	2	0	1	0	1	0	1	-1.108	3
537		3	max	1689.468	3	1395.411	2	0	1	0	1	0	1	1.351	2
538			min	-1067.939	2	-1503.317	3	0	1	0	1	0	1	-2.168	3
539		4	max	1690.084	3	1394.238	2	0	1	0	1	0	1	.615	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1067.117	2	-1504.197	3	0	1	0	1	0	1	-1.374	3
541		5	max	1690.701	3	1393.064	2	0	1	0	1	0	1	0	9
542			min	-1066.296	2	-1505.077	3	0	1	0	1	0	1	-.58	3
543		6	max	1691.317	3	1391.891	2	0	1	0	1	0	1	.214	3
544			min	-1065.474	2	-1505.957	3	0	1	0	1	0	1	-.855	2
545		7	max	1691.933	3	1390.717	2	0	1	0	1	0	1	1.009	3
546			min	-1064.652	2	-1506.837	3	0	1	0	1	0	1	-1.589	2
547		8	max	1692.549	3	1389.544	2	0	1	0	1	0	1	1.804	3
548			min	-1063.831	2	-1507.717	3	0	1	0	1	0	1	-2.323	2
549		9	max	1726.212	3	160.987	2	0	1	0	1	0	1	2.072	3
550			min	-877.585	2	.357	15	0	1	0	1	0	1	-2.649	2
551		10	max	1726.828	3	159.814	2	0	1	0	1	0	1	2.014	3
552			min	-876.764	2	.003	15	0	1	0	1	0	1	-2.733	2
553		11	max	1727.445	3	158.64	2	0	1	0	1	0	1	1.956	3
554			min	-875.942	2	-1.233	4	0	1	0	1	0	1	-2.817	2
555		12	max	1761.254	3	1013.751	3	0	1	0	1	0	1	1.721	3
556			min	-689.719	2	-1717.059	2	0	1	0	1	0	1	-2.525	2
557		13	max	1761.87	3	1012.871	3	0	1	0	1	0	1	1.186	3
558			min	-688.897	2	-1718.233	2	0	1	0	1	0	1	-1.619	2
559		14	max	1762.486	3	1011.991	3	0	1	0	1	0	1	.652	3
560			min	-688.075	2	-1719.406	2	0	1	0	1	0	1	-.712	2
561		15	max	1763.103	3	1011.111	3	0	1	0	1	0	1	.196	2
562			min	-687.254	2	-1720.579	2	0	1	0	1	0	1	-.004	13
563		16	max	1763.719	3	1010.231	3	0	1	0	1	0	1	1.104	2
564			min	-686.432	2	-1721.753	2	0	1	0	1	0	1	-.415	3
565		17	max	1764.335	3	1009.351	3	0	1	0	1	0	1	2.013	2
566			min	-685.61	2	-1722.926	2	0	1	0	1	0	1	-.948	3
567		18	max	-23.41	12	1932.549	2	0	1	0	1	0	1	1.037	2
568			min	-431.974	1	-901.704	3	0	1	0	1	0	1	-.495	3
569		19	max	-22.999	12	1931.376	2	0	1	0	1	0	1	.017	2
570			min	-431.152	1	-902.584	3	0	1	0	1	0	1	-.019	3
571	M9	1	max	201.021	1	627.767	3	140.085	1	0	3	-.019	15	.002	3
572			min	11.138	15	-380.97	2	7.801	15	0	2	-.336	1	-.012	2
573		2	max	201.842	1	626.887	3	140.085	1	0	3	-.015	15	.189	2
574			min	11.386	15	-382.144	2	7.801	15	0	2	-.262	1	-.329	3
575		3	max	523.254	3	452.912	2	139.878	1	0	2	-.01	15	.381	2
576			min	-298.885	2	-461.121	3	7.776	15	0	3	-.188	1	-.647	3
577		4	max	523.87	3	451.739	2	139.878	1	0	2	-.006	15	.148	1
578			min	-298.064	2	-462.001	3	7.776	15	0	3	-.114	1	-.403	3
579		5	max	524.486	3	450.566	2	139.878	1	0	2	-.002	15	-.003	15
580			min	-297.242	2	-462.881	3	7.776	15	0	3	-.041	1	-.159	3
581		6	max	525.102	3	449.392	2	139.878	1	0	2	.033	1	.085	3
582			min	-296.42	2	-463.762	3	7.776	15	0	3	.002	15	-.333	2
583		7	max	525.718	3	448.219	2	139.878	1	0	2	.107	1	.33	3
584			min	-295.599	2	-464.642	3	7.776	15	0	3	.006	15	-.57	2
585		8	max	526.335	3	447.045	2	139.878	1	0	2	.181	1	.576	3
586			min	-294.777	2	-465.522	3	7.776	15	0	3	.01	15	-.806	2
587		9	max	545.006	3	48.231	2	200.499	1	0	3	-.006	15	.671	3
588			min	-203.867	2	.359	15	11.151	15	0	9	-.104	1	-.924	2
589		10	max	545.622	3	47.058	2	200.499	1	0	3	.001	1	.654	3
590			min	-203.045	2	.005	15	11.151	15	0	9	0	15	-.949	2
591		11	max	546.238	3	45.885	2	200.499	1	0	3	.107	1	.638	3
592			min	-202.224	2	-1.413	4	11.151	15	0	9	.006	15	-.974	2
593		12	max	564.836	3	312.736	3	136.68	1	0	3	-.01	15	.556	3
594			min	-117.415	10	-546.334	2	7.589	15	0	2	-.179	1	-.864	2
595		13	max	565.453	3	311.856	3	136.68	1	0	3	-.006	15	.391	3
596			min	-116.73	10	-547.507	2	7.589	15	0	2	-.106	1	-.575	2



Company : Schletter, Inc.  
Designer : HCV  
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Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	566.069	3	310.976	3	136.68	1	0	3	-.002	15	.227	3
598		min	-116.046	10	-548.681	2	7.589	15	0	2	-.034	1	-.286	2
599	15	max	566.685	3	310.096	3	136.68	1	0	3	.038	1	.063	3
600		min	-115.361	10	-549.854	2	7.589	15	0	2	.002	15	-.019	9
601	16	max	567.301	3	309.216	3	136.68	1	0	3	.11	1	.294	2
602		min	-114.676	10	-551.027	2	7.589	15	0	2	.006	15	-.1	3
603	17	max	567.917	3	308.336	3	136.68	1	0	3	.182	1	.585	2
604		min	-113.992	10	-552.201	2	7.589	15	0	2	.01	15	-.263	3
605	18	max	-11.395	15	575.561	2	151.486	1	0	2	.258	1	.295	2
606		min	-202.068	1	-264.882	3	8.428	15	0	3	.014	15	-.13	3
607	19	max	-11.147	15	574.387	2	151.486	1	0	2	.338	1	.01	3
608		min	-201.246	1	-265.762	3	8.428	15	0	3	.019	15	-.009	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.093	2	.009	3	7.922e-3	2	NC	1	NC	1
2			min	0	15	-.014	3	-.005	2	-1.643e-3	3	NC	1	NC	1
3		2	max	.001	1	.408	3	.063	1	9.241e-3	2	NC	5	NC	2
4			min	0	15	-.147	1	.004	15	-1.834e-3	3	639.953	3	4390.176	1
5		3	max	.001	1	.749	3	.154	1	1.056e-2	2	NC	5	NC	3
6			min	0	15	-.331	2	.009	15	-2.025e-3	3	353.713	3	1770.193	1
7		4	max	0	1	.956	3	.233	1	1.188e-2	2	NC	15	NC	3
8			min	0	15	-.438	2	.013	15	-2.217e-3	3	278.288	3	1163.247	1
9		5	max	0	1	1.003	3	.275	1	1.32e-2	2	NC	15	NC	5
10			min	0	15	-.448	2	.016	15	-2.408e-3	3	265.397	3	986.453	1
11		6	max	0	1	.894	3	.266	1	1.452e-2	2	NC	5	NC	5
12			min	0	15	-.366	1	.015	15	-2.599e-3	3	297.25	3	1018.086	1
13		7	max	0	1	.662	3	.21	1	1.584e-2	2	NC	5	NC	5
14			min	0	15	-.218	1	.012	15	-2.79e-3	3	399.641	3	1292.485	1
15		8	max	0	1	.366	3	.123	1	1.716e-2	2	NC	5	NC	5
16			min	0	15	-.037	1	.007	15	-2.981e-3	3	709.666	3	2221.783	1
17		9	max	0	1	.154	2	.035	1	1.847e-2	2	NC	4	NC	2
18			min	0	15	.004	15	-.006	10	-3.173e-3	3	2389.383	3	7888.669	1
19		10	max	0	1	.23	2	.028	3	1.979e-2	2	NC	3	NC	1
20			min	0	1	-.022	3	-.019	2	-3.364e-3	3	1971.705	2	NC	1
21		11	max	0	15	.154	2	.035	1	1.847e-2	2	NC	4	NC	2
22			min	0	1	.004	15	-.006	10	-3.173e-3	3	2389.383	3	7888.669	1
23		12	max	0	15	.366	3	.123	1	1.716e-2	2	NC	5	NC	5
24			min	0	1	-.037	1	.007	15	-2.981e-3	3	709.666	3	2221.783	1
25		13	max	0	15	.662	3	.21	1	1.584e-2	2	NC	5	NC	5
26			min	0	1	-.218	1	.012	15	-2.79e-3	3	399.641	3	1292.485	1
27		14	max	0	15	.894	3	.266	1	1.452e-2	2	NC	5	NC	5
28			min	0	1	-.366	1	.015	15	-2.599e-3	3	297.25	3	1018.086	1
29		15	max	0	15	1.003	3	.275	1	1.32e-2	2	NC	15	NC	5
30			min	0	1	-.448	2	.016	15	-2.408e-3	3	265.397	3	986.453	1
31		16	max	0	15	.956	3	.233	1	1.188e-2	2	NC	15	NC	3
32			min	0	1	-.438	2	.013	15	-2.217e-3	3	278.288	3	1163.247	1
33		17	max	0	15	.749	3	.154	1	1.056e-2	2	NC	5	NC	3
34			min	-.001	1	-.331	2	.009	15	-2.025e-3	3	353.713	3	1770.193	1
35		18	max	0	15	.408	3	.063	1	9.241e-3	2	NC	5	NC	2
36			min	-.001	1	-.147	1	.004	15	-1.834e-3	3	639.953	3	4390.176	1
37		19	max	0	15	.093	2	.009	3	7.922e-3	2	NC	1	NC	1
38			min	-.001	1	-.014	3	-.005	2	-1.643e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.204	3	.008	3	4.674e-3	2	NC	1	NC	1
40			min	0	15	-.311	2	-.004	2	-3.509e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.6	3	.044	1	5.647e-3	2	NC	5	NC	2
42			min	0	15	-.665	2	.003	15	-4.315e-3	3	682.095	3	6295.53	1
43		3	max	0	1	.932	3	.125	1	6.621e-3	2	NC	15	NC	3
44			min	0	15	-.967	2	.007	15	-5.121e-3	3	370.949	3	2175.906	1
45		4	max	0	1	1.157	3	.202	1	7.595e-3	2	NC	15	NC	3
46			min	0	15	-1.182	2	.011	15	-5.927e-3	3	283.424	3	1344.157	1
47		5	max	0	1	1.253	3	.246	1	8.569e-3	2	9173.649	15	NC	5
48			min	0	15	-1.292	2	.014	15	-6.733e-3	3	257.398	3	1102.34	1
49		6	max	0	1	1.222	3	.244	1	9.542e-3	2	9253.711	15	NC	5
50			min	0	15	-1.297	2	.014	15	-7.538e-3	3	265.257	3	1113.868	1
51		7	max	0	1	1.087	3	.195	1	1.052e-2	2	NC	15	NC	5
52			min	0	15	-1.214	2	.011	15	-8.344e-3	3	298.968	2	1393.137	1
53		8	max	0	1	.893	3	.115	1	1.149e-2	2	NC	15	NC	3
54			min	0	15	-1.079	2	.007	15	-9.15e-3	3	351.699	2	2365.814	1
55		9	max	0	1	.708	3	.034	1	1.246e-2	2	NC	5	NC	2
56			min	0	15	-.944	2	-.005	10	-9.956e-3	3	426.699	2	8246.032	1
57		10	max	0	1	.623	3	.025	3	1.344e-2	2	NC	5	NC	1
58			min	0	1	-.88	2	-.018	2	-1.076e-2	3	474.593	2	NC	1
59		11	max	0	15	.708	3	.034	1	1.246e-2	2	NC	5	NC	2
60			min	0	1	-.944	2	-.005	10	-9.956e-3	3	426.699	2	8246.032	1
61		12	max	0	15	.893	3	.115	1	1.149e-2	2	NC	15	NC	3
62			min	0	1	-1.079	2	.007	15	-9.15e-3	3	351.699	2	2365.814	1
63		13	max	0	15	1.087	3	.195	1	1.052e-2	2	NC	15	NC	5
64			min	0	1	-1.214	2	.011	15	-8.344e-3	3	298.968	2	1393.137	1
65		14	max	0	15	1.222	3	.244	1	9.542e-3	2	9253.711	15	NC	5
66			min	0	1	-1.297	2	.014	15	-7.538e-3	3	265.257	3	1113.868	1
67		15	max	0	15	1.253	3	.246	1	8.569e-3	2	9173.649	15	NC	5
68			min	0	1	-1.292	2	.014	15	-6.733e-3	3	257.398	3	1102.34	1
69		16	max	0	15	1.157	3	.202	1	7.595e-3	2	NC	15	NC	3
70			min	0	1	-1.182	2	.011	15	-5.927e-3	3	283.424	3	1344.157	1
71		17	max	0	15	.932	3	.125	1	6.621e-3	2	NC	15	NC	3
72			min	0	1	-.967	2	.007	15	-5.121e-3	3	370.949	3	2175.906	1
73		18	max	0	15	.6	3	.044	1	5.647e-3	2	NC	5	NC	2
74			min	0	1	-.665	2	.003	15	-4.315e-3	3	682.095	3	6295.53	1
75		19	max	0	15	.204	3	.008	3	4.674e-3	2	NC	1	NC	1
76			min	0	1	-.311	2	-.004	2	-3.509e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.207	3	.007	3	3.101e-3	3	NC	1	NC	1
78			min	0	1	-.31	2	-.004	2	-4.913e-3	2	NC	1	NC	1
79		2	max	0	15	.459	3	.044	1	3.821e-3	3	NC	5	NC	2
80			min	0	1	-.793	2	.003	15	-5.942e-3	2	559.134	2	6267.618	1
81		3	max	0	15	.674	3	.126	1	4.54e-3	3	NC	15	NC	3
82			min	0	1	-1.2	2	.007	15	-6.97e-3	2	303.544	2	2170.29	1
83		4	max	0	15	.828	3	.203	1	5.26e-3	3	NC	15	NC	3
84			min	0	1	-1.478	2	.011	15	-7.999e-3	2	231.205	2	1341.482	1
85		5	max	0	15	.909	3	.247	1	5.98e-3	3	9189.477	15	NC	5
86			min	0	1	-1.602	2	.014	15	-9.027e-3	2	208.935	2	1100.364	1
87		6	max	0	15	.916	3	.244	1	6.699e-3	3	9273.053	15	NC	5
88			min	0	1	-1.574	2	.014	15	-1.006e-2	2	213.631	2	1111.81	1
89		7	max	0	15	.861	3	.196	1	7.419e-3	3	NC	15	NC	5
90			min	0	1	-1.42	2	.011	15	-1.108e-2	2	243.239	2	1389.997	1
91		8	max	0	15	.768	3	.116	1	8.139e-3	3	NC	15	NC	3
92			min	0	1	-1.195	2	.007	15	-1.211e-2	2	305.202	2	2357.194	1
93		9	max	0	15	.675	3	.034	1	8.859e-3	3	NC	5	NC	2
94			min	0	1	-.978	2	-.005	10	-1.314e-2	2	404.077	2	8147.128	1
95		10	max	0	1	.631	3	.023	3	9.578e-3	3	NC	5	NC	1
96			min	0	1	-.878	2	-.017	2	-1.417e-2	2	475.777	2	NC	1
97		11	max	0	1	.675	3	.034	1	8.859e-3	3	NC	5	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98			min	0	15	-.978	2	-.005	10	-1.314e-2	2	404.077	2	8147.128	1
99		12	max	0	1	.768	3	.116	1	8.139e-3	3	NC	15	NC	3
100			min	0	15	-1.195	2	.007	15	-1.211e-2	2	305.202	2	2357.194	1
101		13	max	0	1	.861	3	.196	1	7.419e-3	3	NC	15	NC	5
102			min	0	15	-1.42	2	.011	15	-1.108e-2	2	243.239	2	1389.997	1
103		14	max	0	1	.916	3	.244	1	6.699e-3	3	9273.053	15	NC	5
104			min	0	15	-1.574	2	.014	15	-1.006e-2	2	213.631	2	1111.81	1
105		15	max	0	1	.909	3	.247	1	5.98e-3	3	9189.477	15	NC	5
106			min	0	15	-1.602	2	.014	15	-9.027e-3	2	208.935	2	1100.364	1
107		16	max	0	1	.828	3	.203	1	5.26e-3	3	NC	15	NC	3
108			min	0	15	-1.478	2	.011	15	-7.999e-3	2	231.205	2	1341.482	1
109		17	max	0	1	.674	3	.126	1	4.54e-3	3	NC	15	NC	3
110			min	0	15	-1.2	2	.007	15	-6.97e-3	2	303.544	2	2170.29	1
111		18	max	0	1	.459	3	.044	1	3.821e-3	3	NC	5	NC	2
112			min	0	15	-.793	2	.003	15	-5.942e-3	2	559.134	2	6267.618	1
113		19	max	0	1	.207	3	.007	3	3.101e-3	3	NC	1	NC	1
114			min	0	15	-.31	2	-.004	2	-4.913e-3	2	NC	1	NC	1
115	M16	1	max	0	15	.082	2	.006	3	5.318e-3	3	NC	1	NC	1
116			min	-.002	1	-.065	3	-.004	2	-6.434e-3	2	NC	1	NC	1
117		2	max	0	15	.096	3	.062	1	6.375e-3	3	NC	5	NC	2
118			min	-.001	1	-.288	2	.004	15	-7.396e-3	2	729.964	2	4421.732	1
119		3	max	0	15	.224	3	.153	1	7.433e-3	3	NC	5	NC	3
120			min	-.001	1	-.584	2	.009	15	-8.358e-3	2	405.238	2	1776.561	1
121		4	max	0	15	.295	3	.233	1	8.491e-3	3	NC	5	NC	3
122			min	-.001	1	-.758	2	.013	15	-9.32e-3	2	321.44	2	1165.245	1
123		5	max	0	15	.299	3	.275	1	9.548e-3	3	NC	15	NC	5
124			min	0	1	-.786	2	.015	15	-1.028e-2	2	310.975	2	986.696	1
125		6	max	0	15	.237	3	.267	1	1.061e-2	3	NC	5	NC	5
126			min	0	1	-.673	2	.015	15	-1.124e-2	2	357.695	2	1016.665	1
127		7	max	0	15	.126	3	.211	1	1.166e-2	3	NC	5	NC	5
128			min	0	1	-.448	2	.012	15	-1.221e-2	2	509.444	2	1287.301	1
129		8	max	0	15	0	15	.124	1	1.272e-2	3	NC	4	NC	3
130			min	0	1	-.168	2	.007	15	-1.317e-2	2	1078.132	2	2198.802	1
131		9	max	0	15	.097	1	.037	1	1.378e-2	3	NC	1	NC	2
132			min	0	1	-.13	3	-.003	10	-1.413e-2	2	4162.01	3	7540.064	1
133		10	max	0	1	.196	2	.02	3	1.484e-2	3	NC	4	NC	1
134			min	0	1	-.184	3	-.015	2	-1.509e-2	2	2275.42	3	NC	1
135		11	max	0	1	.097	1	.037	1	1.378e-2	3	NC	1	NC	2
136			min	0	15	-.13	3	-.003	10	-1.413e-2	2	4162.01	3	7540.064	1
137		12	max	0	1	0	15	.124	1	1.272e-2	3	NC	4	NC	3
138			min	0	15	-.168	2	.007	15	-1.317e-2	2	1078.132	2	2198.802	1
139		13	max	0	1	.126	3	.211	1	1.166e-2	3	NC	5	NC	5
140			min	0	15	-.448	2	.012	15	-1.221e-2	2	509.444	2	1287.301	1
141		14	max	0	1	.237	3	.267	1	1.061e-2	3	NC	5	NC	5
142			min	0	15	-.673	2	.015	15	-1.124e-2	2	357.695	2	1016.665	1
143		15	max	0	1	.299	3	.275	1	9.548e-3	3	NC	15	NC	5
144			min	0	15	-.786	2	.015	15	-1.028e-2	2	310.975	2	986.696	1
145		16	max	.001	1	.295	3	.233	1	8.491e-3	3	NC	5	NC	3
146			min	0	15	-.758	2	.013	15	-9.32e-3	2	321.44	2	1165.245	1
147		17	max	.001	1	.224	3	.153	1	7.433e-3	3	NC	5	NC	3
148			min	0	15	-.584	2	.009	15	-8.358e-3	2	405.238	2	1776.561	1
149		18	max	.001	1	.096	3	.062	1	6.375e-3	3	NC	5	NC	2
150			min	0	15	-.288	2	.004	15	-7.396e-3	2	729.964	2	4421.732	1
151		19	max	.002	1	.082	2	.006	3	5.318e-3	3	NC	1	NC	1
152			min	0	15	-.065	3	-.004	2	-6.434e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.008	2	.011	1	-1.79e-5	15	NC	1	NC	2
154			min	-.009	3	-.014	3	0	15	-3.222e-4	1	9288.213	2	7113.857	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.007	2	.007	2	.01	1	-1.694e-5	15	NC	1	NC	2
156			min	-.008	3	-.014	3	0	15	-3.048e-4	1	NC	1	7753.155	1
157		3	max	.006	2	.006	2	.009	1	-1.597e-5	15	NC	1	NC	2
158			min	-.008	3	-.013	3	0	15	-2.874e-4	1	NC	1	8514.028	1
159		4	max	.006	2	.005	2	.008	1	-1.5e-5	15	NC	1	NC	2
160			min	-.007	3	-.013	3	0	15	-2.7e-4	1	NC	1	9428.406	1
161		5	max	.005	2	.003	2	.007	1	-1.404e-5	15	NC	1	NC	1
162			min	-.007	3	-.012	3	0	15	-2.526e-4	1	NC	1	NC	1
163		6	max	.005	2	.002	2	.006	1	-1.307e-5	15	NC	1	NC	1
164			min	-.006	3	-.012	3	0	15	-2.352e-4	1	NC	1	NC	1
165		7	max	.005	2	.001	2	.006	1	-1.21e-5	15	NC	1	NC	1
166			min	-.006	3	-.011	3	0	15	-2.178e-4	1	NC	1	NC	1
167		8	max	.004	2	0	2	.005	1	-1.114e-5	15	NC	1	NC	1
168			min	-.005	3	-.011	3	0	15	-2.004e-4	1	NC	1	NC	1
169		9	max	.004	2	0	2	.004	1	-1.017e-5	15	NC	1	NC	1
170			min	-.005	3	-.01	3	0	15	-1.83e-4	1	NC	1	NC	1
171		10	max	.003	2	-.001	2	.003	1	-9.204e-6	15	NC	1	NC	1
172			min	-.004	3	-.009	3	0	15	-1.656e-4	1	NC	1	NC	1
173		11	max	.003	2	-.001	15	.003	1	-8.238e-6	15	NC	1	NC	1
174			min	-.004	3	-.009	3	0	15	-1.482e-4	1	NC	1	NC	1
175		12	max	.003	2	-.001	15	.002	1	-7.272e-6	15	NC	1	NC	1
176			min	-.003	3	-.008	3	0	15	-1.308e-4	1	NC	1	NC	1
177		13	max	.002	2	-.001	15	.002	1	-6.305e-6	15	NC	1	NC	1
178			min	-.003	3	-.007	3	0	15	-1.134e-4	1	NC	1	NC	1
179		14	max	.002	2	-.001	15	.001	1	-5.339e-6	15	NC	1	NC	1
180			min	-.002	3	-.006	3	0	15	-9.602e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-4.373e-6	15	NC	1	NC	1
182			min	-.002	3	-.005	3	0	15	-7.862e-5	1	NC	1	NC	1
183		16	max	.001	2	0	15	0	1	-3.406e-6	15	NC	1	NC	1
184			min	-.001	3	-.004	4	0	15	-6.122e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-2.44e-6	15	NC	1	NC	1
186			min	0	3	-.003	4	0	15	-4.383e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-1.473e-6	15	NC	1	NC	1
188			min	0	3	-.001	4	0	15	-2.643e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-5.071e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-9.036e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	1.378e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	7.802e-8	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	2.925e-5	1	NC	1	NC	1
194			min	0	2	-.002	4	0	1	1.623e-6	15	NC	1	NC	1
195		3	max	0	3	0	15	0	15	5.711e-5	1	NC	1	NC	1
196			min	0	2	-.004	4	0	1	3.168e-6	15	NC	1	NC	1
197		4	max	.001	3	-.001	15	0	1	8.498e-5	1	NC	1	NC	1
198			min	0	2	-.006	4	0	3	4.713e-6	15	NC	1	NC	1
199		5	max	.002	3	-.002	15	0	1	1.128e-4	1	NC	1	NC	1
200			min	-.001	2	-.008	4	0	12	6.258e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	1	1.407e-4	1	NC	1	NC	1
202			min	-.002	2	-.01	4	0	12	7.803e-6	15	9242.098	4	NC	1
203		7	max	.002	3	-.003	15	0	1	1.686e-4	1	NC	1	NC	1
204			min	-.002	2	-.012	4	0	15	9.348e-6	15	7992.855	4	NC	1
205		8	max	.003	3	-.003	15	0	1	1.965e-4	1	NC	2	NC	1
206			min	-.002	2	-.013	4	0	15	1.089e-5	15	7224.093	4	NC	1
207		9	max	.003	3	-.003	15	.001	1	2.243e-4	1	NC	5	NC	1
208			min	-.003	2	-.014	4	0	15	1.244e-5	15	6775.646	4	NC	1
209		10	max	.004	3	-.003	15	.001	1	2.522e-4	1	NC	5	NC	1
210			min	-.003	2	-.014	4	0	15	1.398e-5	15	6570.406	4	NC	1
211		11	max	.004	3	-.003	15	.002	1	2.801e-4	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.003	2	-.014	4	0	15	1.553e-5	15	6578.407	4	NC	1
213		max	.004	3	-.003	15	.003	1	3.079e-4	1	NC	3	NC	1
214		min	-.004	2	-.014	4	0	15	1.707e-5	15	6805.316	4	NC	1
215		max	.005	3	-.003	15	.003	1	3.358e-4	1	NC	2	NC	1
216		min	-.004	2	-.013	4	0	15	1.862e-5	15	7296.104	4	NC	1
217		max	.005	3	-.003	15	.004	1	3.637e-4	1	NC	1	NC	1
218		min	-.004	2	-.012	4	0	15	2.016e-5	15	8157.802	4	NC	1
219		max	.006	3	-.002	15	.005	1	3.915e-4	1	NC	1	NC	1
220		min	-.005	2	-.01	4	0	15	2.171e-5	15	9625.531	4	NC	1
221		max	.006	3	-.002	15	.006	1	4.194e-4	1	NC	1	NC	1
222		min	-.005	2	-.008	4	0	15	2.325e-5	15	NC	1	NC	1
223		max	.006	3	-.001	15	.007	1	4.473e-4	1	NC	1	NC	1
224		min	-.005	2	-.006	4	0	15	2.48e-5	15	NC	1	NC	1
225		max	.007	3	0	15	.009	1	4.751e-4	1	NC	1	NC	1
226		min	-.006	2	-.004	3	0	15	2.634e-5	15	NC	1	NC	1
227		max	.007	3	0	10	.01	1	5.03e-4	1	NC	1	NC	2
228		min	-.006	2	-.002	3	0	15	2.789e-5	15	NC	1	8756.047	1
229	M4	max	.002	1	.006	2	0	15	1.665e-4	1	NC	1	NC	3
230		min	0	15	-.008	3	-.01	1	9.247e-6	15	NC	1	2417.633	1
231		max	.002	1	.005	2	0	15	1.665e-4	1	NC	1	NC	3
232		min	0	15	-.007	3	-.009	1	9.247e-6	15	NC	1	2621.318	1
233		max	.002	1	.005	2	0	15	1.665e-4	1	NC	1	NC	3
234		min	0	15	-.007	3	-.009	1	9.247e-6	15	NC	1	2864.228	1
235		max	.002	1	.005	2	0	15	1.665e-4	1	NC	1	NC	3
236		min	0	15	-.006	3	-.008	1	9.247e-6	15	NC	1	3156.467	1
237		max	.002	1	.004	2	0	15	1.665e-4	1	NC	1	NC	3
238		min	0	15	-.006	3	-.007	1	9.247e-6	15	NC	1	3511.77	1
239		max	.002	1	.004	2	0	15	1.665e-4	1	NC	1	NC	3
240		min	0	15	-.006	3	-.006	1	9.247e-6	15	NC	1	3949.191	1
241		max	.002	1	.004	2	0	15	1.665e-4	1	NC	1	NC	2
242		min	0	15	-.005	3	-.006	1	9.247e-6	15	NC	1	4495.771	1
243		max	.001	1	.003	2	0	15	1.665e-4	1	NC	1	NC	2
244		min	0	15	-.005	3	-.005	1	9.247e-6	15	NC	1	5190.946	1
245		max	.001	1	.003	2	0	15	1.665e-4	1	NC	1	NC	2
246		min	0	15	-.004	3	-.004	1	9.247e-6	15	NC	1	6094.063	1
247		max	.001	1	.003	2	0	15	1.665e-4	1	NC	1	NC	2
248		min	0	15	-.004	3	-.003	1	9.247e-6	15	NC	1	7297.872	1
249		max	.001	1	.003	2	0	15	1.665e-4	1	NC	1	NC	2
250		min	0	15	-.003	3	-.003	1	9.247e-6	15	NC	1	8954.032	1
251		max	0	1	.002	2	0	15	1.665e-4	1	NC	1	NC	1
252		min	0	15	-.003	3	-.002	1	9.247e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	1.665e-4	1	NC	1	NC	1
254		min	0	15	-.003	3	-.002	1	9.247e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	1.665e-4	1	NC	1	NC	1
256		min	0	15	-.002	3	-.001	1	9.247e-6	15	NC	1	NC	1
257		max	0	1	.001	2	0	15	1.665e-4	1	NC	1	NC	1
258		min	0	15	-.002	3	0	1	9.247e-6	15	NC	1	NC	1
259		max	0	1	0	2	0	15	1.665e-4	1	NC	1	NC	1
260		min	0	15	-.001	3	0	1	9.247e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	1.665e-4	1	NC	1	NC	1
262		min	0	15	0	3	0	1	9.247e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	1.665e-4	1	NC	1	NC	1
264		min	0	15	0	3	0	1	9.247e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	1.665e-4	1	NC	1	NC	1
266		min	0	1	0	1	0	1	9.247e-6	15	NC	1	NC	1
267	M6	max	.022	2	.032	2	0	1	0	1	NC	3	NC	1
268		min	-.029	3	-.044	3	0	1	0	1	2421.951	2	NC	1



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.021	2	.029	2	0	1	0	1	NC	3	NC	1
270		min	-.027	3	-.042	3	0	1	0	1	2662.893	2	NC	1
271	3	max	.02	2	.026	2	0	1	0	1	NC	3	NC	1
272		min	-.026	3	-.04	3	0	1	0	1	2954.251	2	NC	1
273	4	max	.019	2	.023	2	0	1	0	1	NC	3	NC	1
274		min	-.024	3	-.037	3	0	1	0	1	3310.256	2	NC	1
275	5	max	.017	2	.021	2	0	1	0	1	NC	3	NC	1
276		min	-.023	3	-.035	3	0	1	0	1	3750.715	2	NC	1
277	6	max	.016	2	.018	2	0	1	0	1	NC	1	NC	1
278		min	-.021	3	-.032	3	0	1	0	1	4303.843	2	NC	1
279	7	max	.015	2	.015	2	0	1	0	1	NC	1	NC	1
280		min	-.019	3	-.03	3	0	1	0	1	5010.939	2	NC	1
281	8	max	.014	2	.013	2	0	1	0	1	NC	1	NC	1
282		min	-.018	3	-.027	3	0	1	0	1	5934.41	2	NC	1
283	9	max	.012	2	.011	2	0	1	0	1	NC	1	NC	1
284		min	-.016	3	-.025	3	0	1	0	1	7172.228	2	NC	1
285	10	max	.011	2	.009	2	0	1	0	1	NC	1	NC	1
286		min	-.015	3	-.022	3	0	1	0	1	8885.445	2	NC	1
287	11	max	.01	2	.007	2	0	1	0	1	NC	1	NC	1
288		min	-.013	3	-.02	3	0	1	0	1	NC	1	NC	1
289	12	max	.009	2	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.011	3	-.017	3	0	1	0	1	NC	1	NC	1
291	13	max	.007	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.01	3	-.015	3	0	1	0	1	NC	1	NC	1
293	14	max	.006	2	.002	2	0	1	0	1	NC	1	NC	1
294		min	-.008	3	-.012	3	0	1	0	1	NC	1	NC	1
295	15	max	.005	2	.001	2	0	1	0	1	NC	1	NC	1
296		min	-.006	3	-.01	3	0	1	0	1	NC	1	NC	1
297	16	max	.004	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.007	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	0	15	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.005	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.006	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.006	2	-.012	3	0	1	0	1	9146.797	3	NC	1
317	7	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.013	3	0	1	0	1	8153.517	4	NC	1
319	8	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7359.593	4	NC	1
321	9	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.01	2	-.016	3	0	1	0	1	6895.086	4	NC	1
323	10	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.011	2	-.016	3	0	1	0	1	6679.96	4	NC	1
325	11	max	.013	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.016	3	0	1	0	1	6682.772	4	NC	1
327		12	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.014	2	-.016	3	0	1	0	1	6908.611	4	NC	1
329		13	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.015	2	-.016	3	0	1	0	1	7402.624	4	NC	1
331		14	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.016	2	-.015	3	0	1	0	1	8272.958	4	NC	1
333		15	max	.018	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.018	2	-.014	3	0	1	0	1	9757.597	4	NC	1
335		16	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.019	2	-.012	3	0	1	0	1	NC	1	NC	1
337		17	max	.02	3	-.001	10	0	1	0	1	NC	1	NC	1
338			min	-.02	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.022	3	0	10	0	1	0	1	NC	1	NC	1
340			min	-.021	2	-.009	3	0	1	0	1	NC	1	NC	1
341		19	max	.023	3	.002	2	0	1	0	1	NC	1	NC	1
342			min	-.023	2	-.007	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.006	1	.022	2	0	1	0	1	NC	1	NC	1
344			min	0	15	-.024	3	0	1	0	1	NC	1	NC	1
345		2	max	.005	1	.021	2	0	1	0	1	NC	1	NC	1
346			min	0	15	-.023	3	0	1	0	1	NC	1	NC	1
347		3	max	.005	1	.02	2	0	1	0	1	NC	1	NC	1
348			min	0	15	-.021	3	0	1	0	1	NC	1	NC	1
349		4	max	.005	1	.018	2	0	1	0	1	NC	1	NC	1
350			min	0	15	-.02	3	0	1	0	1	NC	1	NC	1
351		5	max	.004	1	.017	2	0	1	0	1	NC	1	NC	1
352			min	0	15	-.019	3	0	1	0	1	NC	1	NC	1
353		6	max	.004	1	.016	2	0	1	0	1	NC	1	NC	1
354			min	0	15	-.017	3	0	1	0	1	NC	1	NC	1
355		7	max	.004	1	.015	2	0	1	0	1	NC	1	NC	1
356			min	0	15	-.016	3	0	1	0	1	NC	1	NC	1
357		8	max	.004	1	.014	2	0	1	0	1	NC	1	NC	1
358			min	0	15	-.015	3	0	1	0	1	NC	1	NC	1
359		9	max	.003	1	.012	2	0	1	0	1	NC	1	NC	1
360			min	0	15	-.013	3	0	1	0	1	NC	1	NC	1
361		10	max	.003	1	.011	2	0	1	0	1	NC	1	NC	1
362			min	0	15	-.012	3	0	1	0	1	NC	1	NC	1
363		11	max	.003	1	.01	2	0	1	0	1	NC	1	NC	1
364			min	0	15	-.011	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	1	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	15	-.009	3	0	1	0	1	NC	1	NC	1
367		13	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
368			min	0	15	-.008	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	1	.006	2	0	1	0	1	NC	1	NC	1
370			min	0	15	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	.001	1	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	15	-.005	3	0	1	0	1	NC	1	NC	1
373		16	max	0	1	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	15	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	15	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	15	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.008	2	0	15	3.222e-4	1	NC	1	NC	2
382			min	-.009	3	-.014	3	-.011	1	1.79e-5	15	9288.213	2	7113.857	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	2	.007	2	0	15	3.048e-4	1	NC	1	NC	2
384		min	-.008	3	-.014	3	-.01	1	1.694e-5	15	NC	1	7753.155	1
385	3	max	.006	2	.006	2	0	15	2.874e-4	1	NC	1	NC	2
386		min	-.008	3	-.013	3	-.009	1	1.597e-5	15	NC	1	8514.028	1
387	4	max	.006	2	.005	2	0	15	2.7e-4	1	NC	1	NC	2
388		min	-.007	3	-.013	3	-.008	1	1.5e-5	15	NC	1	9428.406	1
389	5	max	.005	2	.003	2	0	15	2.526e-4	1	NC	1	NC	1
390		min	-.007	3	-.012	3	-.007	1	1.404e-5	15	NC	1	NC	1
391	6	max	.005	2	.002	2	0	15	2.352e-4	1	NC	1	NC	1
392		min	-.006	3	-.012	3	-.006	1	1.307e-5	15	NC	1	NC	1
393	7	max	.005	2	.001	2	0	15	2.178e-4	1	NC	1	NC	1
394		min	-.006	3	-.011	3	-.006	1	1.21e-5	15	NC	1	NC	1
395	8	max	.004	2	0	2	0	15	2.004e-4	1	NC	1	NC	1
396		min	-.005	3	-.011	3	-.005	1	1.114e-5	15	NC	1	NC	1
397	9	max	.004	2	0	2	0	15	1.83e-4	1	NC	1	NC	1
398		min	-.005	3	-.01	3	-.004	1	1.017e-5	15	NC	1	NC	1
399	10	max	.003	2	-.001	2	0	15	1.656e-4	1	NC	1	NC	1
400		min	-.004	3	-.009	3	-.003	1	9.204e-6	15	NC	1	NC	1
401	11	max	.003	2	-.001	15	0	15	1.482e-4	1	NC	1	NC	1
402		min	-.004	3	-.009	3	-.003	1	8.238e-6	15	NC	1	NC	1
403	12	max	.003	2	-.001	15	0	15	1.308e-4	1	NC	1	NC	1
404		min	-.003	3	-.008	3	-.002	1	7.272e-6	15	NC	1	NC	1
405	13	max	.002	2	-.001	15	0	15	1.134e-4	1	NC	1	NC	1
406		min	-.003	3	-.007	3	-.002	1	6.305e-6	15	NC	1	NC	1
407	14	max	.002	2	-.001	15	0	15	9.602e-5	1	NC	1	NC	1
408		min	-.002	3	-.006	3	-.001	1	5.339e-6	15	NC	1	NC	1
409	15	max	.002	2	-.001	15	0	15	7.862e-5	1	NC	1	NC	1
410		min	-.002	3	-.005	3	0	1	4.373e-6	15	NC	1	NC	1
411	16	max	.001	2	0	15	0	15	6.122e-5	1	NC	1	NC	1
412		min	-.001	3	-.004	4	0	1	3.406e-6	15	NC	1	NC	1
413	17	max	0	2	0	15	0	15	4.383e-5	1	NC	1	NC	1
414		min	0	3	-.003	4	0	1	2.44e-6	15	NC	1	NC	1
415	18	max	0	2	0	15	0	15	2.643e-5	1	NC	1	NC	1
416		min	0	3	-.001	4	0	1	1.473e-6	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	9.036e-6	1	NC	1	NC	1
418		min	0	1	0	1	0	1	5.071e-7	15	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-7.802e-8	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-1.378e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-1.623e-6	15	NC	1	NC	1
422		min	0	2	-.002	4	0	15	-2.925e-5	1	NC	1	NC	1
423	3	max	0	3	0	15	0	1	-3.168e-6	15	NC	1	NC	1
424		min	0	2	-.004	4	0	15	-5.711e-5	1	NC	1	NC	1
425	4	max	.001	3	-.001	15	0	3	-4.713e-6	15	NC	1	NC	1
426		min	0	2	-.006	4	0	1	-8.498e-5	1	NC	1	NC	1
427	5	max	.002	3	-.002	15	0	12	-6.258e-6	15	NC	1	NC	1
428		min	-.001	2	-.008	4	0	1	-1.128e-4	1	NC	1	NC	1
429	6	max	.002	3	-.002	15	0	12	-7.803e-6	15	NC	1	NC	1
430		min	-.002	2	-.01	4	0	1	-1.407e-4	1	9242.098	4	NC	1
431	7	max	.002	3	-.003	15	0	15	-9.348e-6	15	NC	1	NC	1
432		min	-.002	2	-.012	4	0	1	-1.686e-4	1	7992.855	4	NC	1
433	8	max	.003	3	-.003	15	0	15	-1.089e-5	15	NC	2	NC	1
434		min	-.002	2	-.013	4	0	1	-1.965e-4	1	7224.093	4	NC	1
435	9	max	.003	3	-.003	15	0	15	-1.244e-5	15	NC	5	NC	1
436		min	-.003	2	-.014	4	-.001	1	-2.243e-4	1	6775.646	4	NC	1
437	10	max	.004	3	-.003	15	0	15	-1.398e-5	15	NC	5	NC	1
438		min	-.003	2	-.014	4	-.001	1	-2.522e-4	1	6570.406	4	NC	1
439	11	max	.004	3	-.003	15	0	15	-1.553e-5	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.003	2	-.014	4	-.002	1	-2.801e-4	1	6578.407	4	NC	1
441		max	.004	3	-.003	15	0	15	-1.707e-5	15	NC	3	NC	1
442		min	-.004	2	-.014	4	-.003	1	-3.079e-4	1	6805.316	4	NC	1
443		max	.005	3	-.003	15	0	15	-1.862e-5	15	NC	2	NC	1
444		min	-.004	2	-.013	4	-.003	1	-3.358e-4	1	7296.104	4	NC	1
445		max	.005	3	-.003	15	0	15	-2.016e-5	15	NC	1	NC	1
446		min	-.004	2	-.012	4	-.004	1	-3.637e-4	1	8157.802	4	NC	1
447		max	.006	3	-.002	15	0	15	-2.171e-5	15	NC	1	NC	1
448		min	-.005	2	-.01	4	-.005	1	-3.915e-4	1	9625.531	4	NC	1
449		max	.006	3	-.002	15	0	15	-2.325e-5	15	NC	1	NC	1
450		min	-.005	2	-.008	4	-.006	1	-4.194e-4	1	NC	1	NC	1
451		max	.006	3	-.001	15	0	15	-2.48e-5	15	NC	1	NC	1
452		min	-.005	2	-.006	4	-.007	1	-4.473e-4	1	NC	1	NC	1
453		max	.007	3	0	15	0	15	-2.634e-5	15	NC	1	NC	1
454		min	-.006	2	-.004	3	-.009	1	-4.751e-4	1	NC	1	NC	1
455		max	.007	3	0	10	0	15	-2.789e-5	15	NC	1	NC	2
456		min	-.006	2	-.002	3	-.01	1	-5.03e-4	1	NC	1	8756.047	1
457	M12	max	.002	1	.006	2	.01	1	-9.247e-6	15	NC	1	NC	3
458		min	0	15	-.008	3	0	15	-1.665e-4	1	NC	1	2417.633	1
459		max	.002	1	.005	2	.009	1	-9.247e-6	15	NC	1	NC	3
460		min	0	15	-.007	3	0	15	-1.665e-4	1	NC	1	2621.318	1
461		max	.002	1	.005	2	.009	1	-9.247e-6	15	NC	1	NC	3
462		min	0	15	-.007	3	0	15	-1.665e-4	1	NC	1	2864.228	1
463		max	.002	1	.005	2	.008	1	-9.247e-6	15	NC	1	NC	3
464		min	0	15	-.006	3	0	15	-1.665e-4	1	NC	1	3156.467	1
465		max	.002	1	.004	2	.007	1	-9.247e-6	15	NC	1	NC	3
466		min	0	15	-.006	3	0	15	-1.665e-4	1	NC	1	3511.77	1
467		max	.002	1	.004	2	.006	1	-9.247e-6	15	NC	1	NC	3
468		min	0	15	-.006	3	0	15	-1.665e-4	1	NC	1	3949.191	1
469		max	.002	1	.004	2	.006	1	-9.247e-6	15	NC	1	NC	2
470		min	0	15	-.005	3	0	15	-1.665e-4	1	NC	1	4495.771	1
471		max	.001	1	.003	2	.005	1	-9.247e-6	15	NC	1	NC	2
472		min	0	15	-.005	3	0	15	-1.665e-4	1	NC	1	5190.946	1
473		max	.001	1	.003	2	.004	1	-9.247e-6	15	NC	1	NC	2
474		min	0	15	-.004	3	0	15	-1.665e-4	1	NC	1	6094.063	1
475		max	.001	1	.003	2	.003	1	-9.247e-6	15	NC	1	NC	2
476		min	0	15	-.004	3	0	15	-1.665e-4	1	NC	1	7297.872	1
477		max	.001	1	.003	2	.003	1	-9.247e-6	15	NC	1	NC	2
478		min	0	15	-.003	3	0	15	-1.665e-4	1	NC	1	8954.032	1
479		max	0	1	.002	2	.002	1	-9.247e-6	15	NC	1	NC	1
480		min	0	15	-.003	3	0	15	-1.665e-4	1	NC	1	NC	1
481		max	0	1	.002	2	.002	1	-9.247e-6	15	NC	1	NC	1
482		min	0	15	-.003	3	0	15	-1.665e-4	1	NC	1	NC	1
483		max	0	1	.002	2	.001	1	-9.247e-6	15	NC	1	NC	1
484		min	0	15	-.002	3	0	15	-1.665e-4	1	NC	1	NC	1
485		max	0	1	.001	2	0	1	-9.247e-6	15	NC	1	NC	1
486		min	0	15	-.002	3	0	15	-1.665e-4	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-9.247e-6	15	NC	1	NC	1
488		min	0	15	-.001	3	0	15	-1.665e-4	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-9.247e-6	15	NC	1	NC	1
490		min	0	15	0	3	0	15	-1.665e-4	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-9.247e-6	15	NC	1	NC	1
492		min	0	15	0	3	0	15	-1.665e-4	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-9.247e-6	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-1.665e-4	1	NC	1	NC	1
495	M1	max	.009	3	.093	2	.001	1	1.643e-2	2	NC	1	NC	1
496		min	-.005	2	-.014	3	0	15	-2.917e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.009	3	.043	2	0	15	8.05e-3	2	NC	4	NC	1
498			min	-.005	2	-.003	3	-.007	1	-1.443e-2	3	2290.872	2	NC	1
499		3	max	.009	3	.014	3	0	15	1.017e-5	10	NC	5	NC	2
500			min	-.005	2	-.011	2	-.011	1	-2.284e-4	1	1103.275	2	9497.737	1
501		4	max	.009	3	.044	3	0	15	4.465e-3	2	NC	5	NC	1
502			min	-.005	2	-.072	2	-.01	1	-5.161e-3	3	695.729	2	NC	1
503		5	max	.009	3	.082	3	0	15	8.966e-3	2	NC	5	NC	1
504			min	-.004	2	-.136	2	-.007	1	-1.017e-2	3	501.68	2	NC	1
505		6	max	.008	3	.123	3	0	15	1.347e-2	2	NC	15	NC	1
506			min	-.004	2	-.198	2	-.003	1	-1.519e-2	3	394.861	2	NC	1
507		7	max	.008	3	.162	3	0	1	1.797e-2	2	NC	15	NC	1
508			min	-.004	2	-.254	2	0	12	-2.02e-2	3	331.848	2	NC	1
509		8	max	.008	3	.194	3	.001	1	2.247e-2	2	9430.853	15	NC	1
510			min	-.004	2	-.298	2	0	15	-2.521e-2	3	294.597	2	NC	1
511		9	max	.008	3	.215	3	0	15	2.594e-2	2	8808.559	15	NC	1
512			min	-.004	2	-.326	2	0	1	-2.534e-2	3	275.215	2	NC	1
513		10	max	.008	3	.223	3	0	1	2.872e-2	2	8619.186	15	NC	1
514			min	-.004	2	-.335	2	0	12	-2.222e-2	3	269.554	2	NC	1
515		11	max	.008	3	.217	3	0	1	3.15e-2	2	8808.223	15	NC	1
516			min	-.004	2	-.325	2	0	15	-1.91e-2	3	276.261	2	NC	1
517		12	max	.007	3	.199	3	0	15	3.075e-2	2	9430.155	15	NC	1
518			min	-.004	2	-.296	2	-.001	1	-1.596e-2	3	297.811	2	NC	1
519		13	max	.007	3	.169	3	0	15	2.468e-2	2	NC	15	NC	1
520			min	-.004	2	-.249	2	0	1	-1.277e-2	3	339.736	2	NC	1
521		14	max	.007	3	.131	3	.003	1	1.86e-2	2	NC	15	NC	1
522			min	-.004	2	-.191	2	0	15	-9.584e-3	3	411.82	2	NC	1
523		15	max	.007	3	.09	3	.007	1	1.253e-2	2	NC	5	NC	1
524			min	-.004	2	-.128	2	0	15	-6.396e-3	3	536.808	2	NC	1
525		16	max	.007	3	.046	3	.01	1	6.456e-3	2	NC	5	NC	1
526			min	-.004	2	-.064	2	0	15	-3.208e-3	3	770.353	2	NC	1
527		17	max	.006	3	.005	3	.01	1	6.748e-4	1	NC	5	NC	2
528			min	-.004	2	-.006	2	0	15	-2.038e-5	3	1274.014	2	9867.435	1
529		18	max	.006	3	.041	2	.007	1	1.278e-2	2	NC	4	NC	1
530			min	-.004	2	-.031	3	0	15	-5.512e-3	3	2728.366	2	NC	1
531		19	max	.006	3	.082	2	0	15	2.561e-2	2	NC	1	NC	1
532			min	-.004	2	-.065	3	-.002	1	-1.121e-2	3	NC	1	NC	1
533	M5	1	max	.028	3	.23	2	0	1	0	1	NC	1	NC	1
534			min	-.019	2	-.022	3	0	1	0	1	NC	1	NC	1
535		2	max	.028	3	.104	2	0	1	0	1	NC	5	NC	1
536			min	-.019	2	.001	3	0	1	0	1	913.517	2	NC	1
537		3	max	.028	3	.047	3	0	1	0	1	NC	5	NC	1
538			min	-.019	2	-.037	2	0	1	0	1	432.309	2	NC	1
539		4	max	.028	3	.133	3	0	1	0	1	9709.607	15	NC	1
540			min	-.019	2	-.203	2	0	1	0	1	266.666	2	NC	1
541		5	max	.027	3	.246	3	0	1	0	1	6801.9	15	NC	1
542			min	-.019	2	-.381	2	0	1	0	1	188.867	2	NC	1
543		6	max	.026	3	.371	3	0	1	0	1	5240.636	15	NC	1
544			min	-.018	2	-.556	2	0	1	0	1	146.664	2	NC	1
545		7	max	.026	3	.491	3	0	1	0	1	4338.225	15	NC	1
546			min	-.018	2	-.715	2	0	1	0	1	122.07	2	NC	1
547		8	max	.025	3	.592	3	0	1	0	1	3813.176	15	NC	1
548			min	-.018	2	-.842	2	0	1	0	1	107.675	2	NC	1
549		9	max	.025	3	.656	3	0	1	0	1	3543.788	15	NC	1
550			min	-.017	2	-.922	2	0	1	0	1	100.256	2	NC	1
551		10	max	.024	3	.679	3	0	1	0	1	3462.62	15	NC	1
552			min	-.017	2	-.949	2	0	1	0	1	98.092	2	NC	1
553		11	max	.024	3	.661	3	0	1	0	1	3543.899	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.017	2	-.921	2	0	1	0	1	100.649	2	NC	1
555		12	max	.023	3	.604	3	0	1	0	1	3813.444	15	NC	1
556			min	-.017	2	-.837	2	0	1	0	1	108.951	2	NC	1
557		13	max	.022	3	.512	3	0	1	0	1	4338.791	15	NC	1
558			min	-.016	2	-.702	2	0	1	0	1	125.358	2	NC	1
559		14	max	.022	3	.397	3	0	1	0	1	5241.772	15	NC	1
560			min	-.016	2	-.535	2	0	1	0	1	154.031	2	NC	1
561		15	max	.021	3	.269	3	0	1	0	1	6804.186	15	NC	1
562			min	-.016	2	-.353	2	0	1	0	1	204.811	2	NC	1
563		16	max	.021	3	.138	3	0	1	0	1	9714.445	15	NC	1
564			min	-.016	2	-.175	2	0	1	0	1	302.37	2	NC	1
565		17	max	.02	3	.016	3	0	1	0	1	NC	5	NC	1
566			min	-.015	2	-.02	2	0	1	0	1	519.345	2	NC	1
567		18	max	.02	3	.098	2	0	1	0	1	NC	5	NC	1
568			min	-.015	2	-.089	3	0	1	0	1	1147.445	2	NC	1
569		19	max	.02	3	.196	2	0	1	0	1	NC	1	NC	1
570			min	-.015	2	-.184	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.009	3	.093	2	0	15	2.917e-2	3	NC	1	NC	1
572			min	-.005	2	-.014	3	-.001	1	-1.643e-2	2	NC	1	NC	1
573		2	max	.009	3	.043	2	.007	1	1.443e-2	3	NC	4	NC	1
574			min	-.005	2	-.003	3	0	15	-8.05e-3	2	2290.872	2	NC	1
575		3	max	.009	3	.014	3	.011	1	2.284e-4	1	NC	5	NC	2
576			min	-.005	2	-.011	2	0	15	-1.017e-5	10	1103.275	2	9497.737	1
577		4	max	.009	3	.044	3	.01	1	5.161e-3	3	NC	5	NC	1
578			min	-.005	2	-.072	2	0	15	-4.465e-3	2	695.729	2	NC	1
579		5	max	.009	3	.082	3	.007	1	1.017e-2	3	NC	5	NC	1
580			min	-.004	2	-.136	2	0	15	-8.966e-3	2	501.68	2	NC	1
581		6	max	.008	3	.123	3	.003	1	1.519e-2	3	NC	15	NC	1
582			min	-.004	2	-.198	2	0	15	-1.347e-2	2	394.861	2	NC	1
583		7	max	.008	3	.162	3	0	12	2.02e-2	3	NC	15	NC	1
584			min	-.004	2	-.254	2	0	1	-1.797e-2	2	331.848	2	NC	1
585		8	max	.008	3	.194	3	0	15	2.521e-2	3	9430.853	15	NC	1
586			min	-.004	2	-.298	2	-.001	1	-2.247e-2	2	294.597	2	NC	1
587		9	max	.008	3	.215	3	0	1	2.534e-2	3	8808.559	15	NC	1
588			min	-.004	2	-.326	2	0	15	-2.594e-2	2	275.215	2	NC	1
589		10	max	.008	3	.223	3	0	12	2.222e-2	3	8619.186	15	NC	1
590			min	-.004	2	-.335	2	0	1	-2.872e-2	2	269.554	2	NC	1
591		11	max	.008	3	.217	3	0	15	1.91e-2	3	8808.223	15	NC	1
592			min	-.004	2	-.325	2	0	1	-3.15e-2	2	276.261	2	NC	1
593		12	max	.007	3	.199	3	.001	1	1.596e-2	3	9430.155	15	NC	1
594			min	-.004	2	-.296	2	0	15	-3.075e-2	2	297.811	2	NC	1
595		13	max	.007	3	.169	3	0	1	1.277e-2	3	NC	15	NC	1
596			min	-.004	2	-.249	2	0	15	-2.468e-2	2	339.736	2	NC	1
597		14	max	.007	3	.131	3	0	15	9.584e-3	3	NC	15	NC	1
598			min	-.004	2	-.191	2	-.003	1	-1.86e-2	2	411.82	2	NC	1
599		15	max	.007	3	.09	3	0	15	6.396e-3	3	NC	5	NC	1
600			min	-.004	2	-.128	2	-.007	1	-1.253e-2	2	536.808	2	NC	1
601		16	max	.007	3	.046	3	0	15	3.208e-3	3	NC	5	NC	1
602			min	-.004	2	-.064	2	-.01	1	-6.456e-3	2	770.353	2	NC	1
603		17	max	.006	3	.005	3	0	15	2.038e-5	3	NC	5	NC	2
604			min	-.004	2	-.006	2	-.01	1	-6.748e-4	1	1274.014	2	9867.435	1
605		18	max	.006	3	.041	2	0	15	5.512e-3	3	NC	4	NC	1
606			min	-.004	2	-.031	3	-.007	1	-1.278e-2	2	2728.366	2	NC	1
607		19	max	.006	3	.082	2	.002	1	1.121e-2	3	NC	1	NC	1
608			min	-.004	2	-.065	3	0	15	-2.561e-2	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1723  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

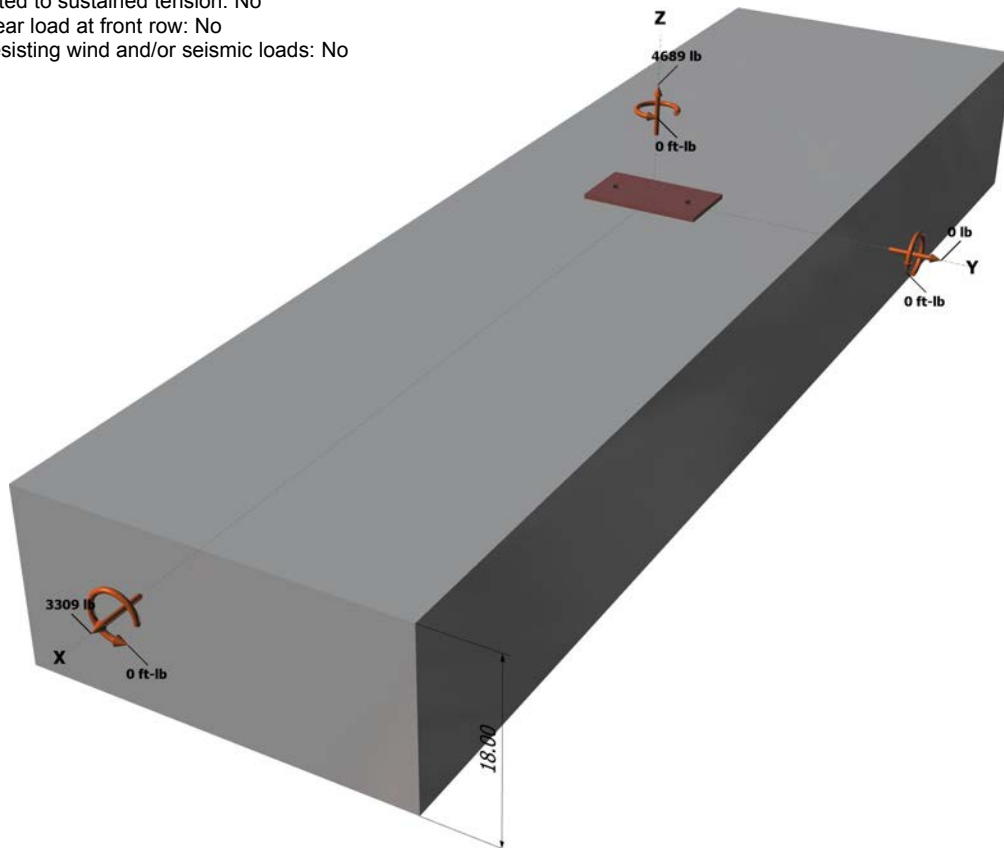
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

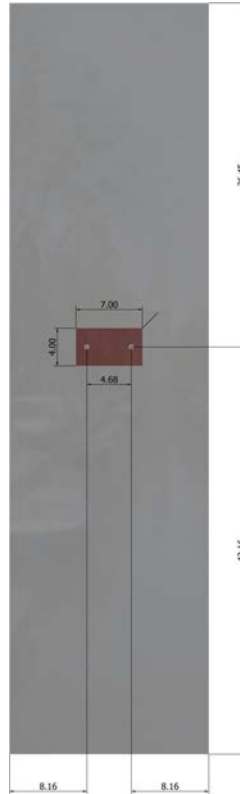
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2344.5	1654.5	0.0	1654.5
2	2344.5	1654.5	0.0	1654.5
Sum	4689.0	3309.0	0.0	3309.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 4689  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cpq} \text{ (lb)}$$

19833

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2345	6071	0.39	Pass	
Concrete breakout	4689	9208	0.51	Pass	
<b>Adhesive</b>	<b>4689</b>	<b>8093</b>	<b>0.58</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
Steel	1655	3156	0.52	Pass	
<b>T Concrete breakout x+</b>	<b>3309</b>	<b>5323</b>	<b>0.62</b>	<b>Pass (Governs)</b>	
<b>   Concrete breakout y-</b>	<b>1655</b>	<b>12241</b>	<b>0.14</b>	<b>Pass (Governs)</b>	
Pryout	3309	19833	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 21-30 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.58	0.62	120.1 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.