

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	110 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 19.00 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

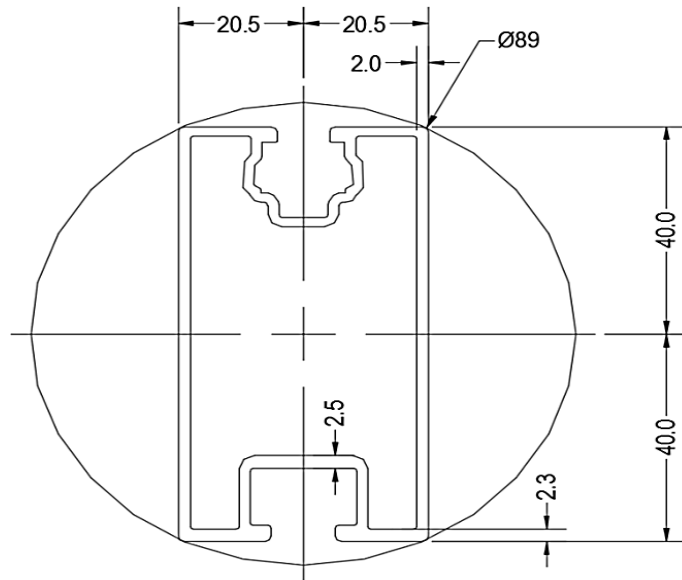
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	60 in
ΦF_{ty} STRONG-AXIS =	29.75 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.484 k-ft
M_z =	0.088 k-ft
$M_{y \text{ allowable}}$ =	1.848 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	37%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.76 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.474 k-ft
M_z =	0.000 k-ft
P_n =	0.261 k
$M_{y \text{ allowable}}$ =	1.460 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	35%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.032 k-ft
P_n =	0.191 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	9%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.320 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.856 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	13%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.189 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	8%



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

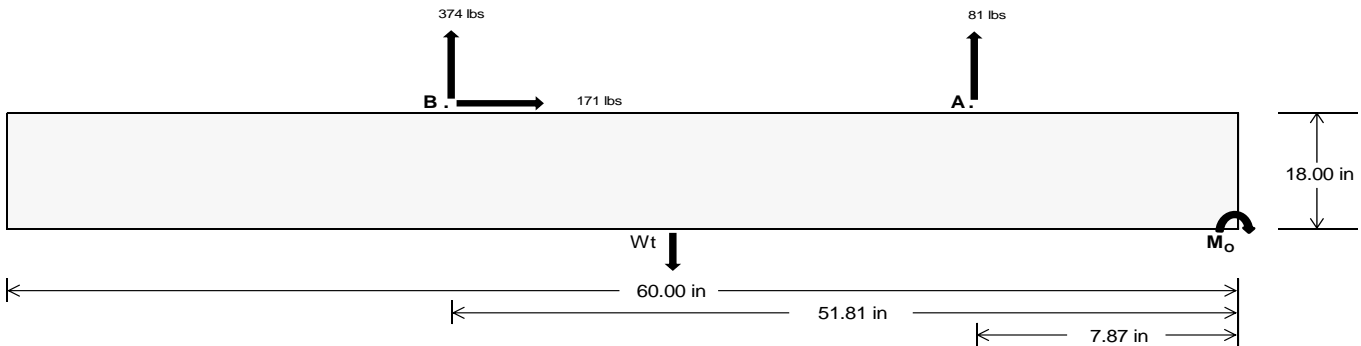
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	340.87	1557.27	k
Compressive Load =	1413.84	1097.06	k
Lateral Load =	26.19	710.55	k
Moment (Weak Axis) =	0.04	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23077.1$ in-lbs
Resisting Force Required = 769.24 lbs
S.F. = 1.67
Weight Required = 1282.06 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 170.78 lbs
Friction = 0.4
Weight Required = 426.95 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 170.78 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	481 lbs	481 lbs	481 lbs	481 lbs	508 lbs	508 lbs	508 lbs	508 lbs	705 lbs	705 lbs	705 lbs	705 lbs	-162 lbs	-162 lbs	-162 lbs	-162 lbs
F_B	344 lbs	344 lbs	344 lbs	344 lbs	448 lbs	448 lbs	448 lbs	448 lbs	567 lbs	567 lbs	567 lbs	567 lbs	-748 lbs	-748 lbs	-748 lbs	-748 lbs
F_V	34 lbs	34 lbs	34 lbs	34 lbs	302 lbs	302 lbs	302 lbs	302 lbs	250 lbs	250 lbs	250 lbs	250 lbs	-342 lbs	-342 lbs	-342 lbs	-342 lbs
P_{total}	2728 lbs	2818 lbs	2909 lbs	3000 lbs	2859 lbs	2950 lbs	3040 lbs	3131 lbs	3175 lbs	3266 lbs	3356 lbs	3447 lbs	232 lbs	287 lbs	341 lbs	395 lbs
M	312 lbs-ft	312 lbs-ft	312 lbs-ft	312 lbs-ft	576 lbs-ft	576 lbs-ft	576 lbs-ft	576 lbs-ft	644 lbs-ft	644 lbs-ft	644 lbs-ft	644 lbs-ft	548 lbs-ft	548 lbs-ft	548 lbs-ft	548 lbs-ft
e	0.11 ft	0.11 ft	0.11 ft	0.10 ft	0.20 ft	0.20 ft	0.19 ft	0.18 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	2.36 ft	1.91 ft	1.61 ft	1.38 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	269.0 psf	266.6 psf	264.5 psf	262.5 psf	247.7 psf	246.4 psf	245.1 psf	244.0 psf	274.5 psf	271.9 psf	269.5 psf	267.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	354.5 psf	348.3 psf	342.6 psf	337.4 psf	405.7 psf	397.2 psf	389.4 psf	382.2 psf	451.2 psf	440.6 psf	430.9 psf	422.0 psf	619.4 psf	176.7 psf	132.6 psf	118.2 psf

Maximum Bearing Pressure = 619 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

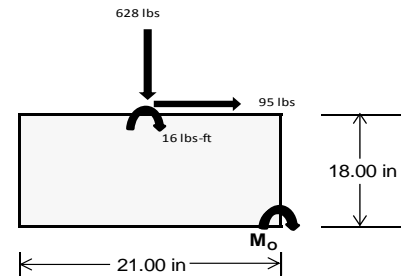
Overturning Check

$M_o = 390.4 \text{ ft-lbs}$
 Resisting Force Required = 446.18 lbs
 S.F. = 1.67
 Weight Required = 743.63 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	116 lbs	98 lbs	62 lbs	270 lbs	628 lbs	228 lbs	73 lbs	-13 lbs	21 lbs
F_v	15 lbs	127 lbs	15 lbs	10 lbs	95 lbs	11 lbs	15 lbs	127 lbs	15 lbs
P_{total}	2472 lbs	2454 lbs	2418 lbs	2512 lbs	2871 lbs	2470 lbs	762 lbs	676 lbs	710 lbs
M	42 lbs-ft	211 lbs-ft	43 lbs-ft	29 lbs-ft	159 lbs-ft	33 lbs-ft	42 lbs-ft	211 lbs-ft	43 lbs-ft
e	0.02 ft	0.09 ft	0.02 ft	0.01 ft	0.06 ft	0.01 ft	0.06 ft	0.31 ft	0.06 ft
$L/6$	0.29 ft	1.58 ft	1.71 ft	1.73 ft	1.64 ft	1.72 ft	1.64 ft	1.13 ft	1.63 ft
f_{min}	266.1 sqft	197.7 sqft	259.4 sqft	275.6 sqft	265.7 sqft	269.5 sqft	70.6 sqft	-5.5 sqft	64.3 sqft
f_{max}	299.0 psf	363.2 psf	293.2 psf	298.6 psf	390.5 psf	295.2 psf	103.6 psf	159.9 psf	98.0 psf



Maximum Bearing Pressure = 391 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

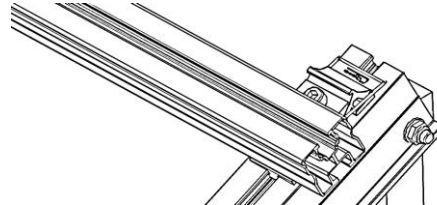
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.604 k
Allowable Uplift =	1.214 k
Utilization =	<u>50%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.072 k
Allowable Uplift =	1.116 k
Utilization =	<u>96%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.088 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

Diagonal Strut

Maximum Axial Load =	0.320 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.115 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.189 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.066 in
	<u>0.066 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 60.00 \text{ in}$$

$$J = 0.427$$

$$125.139$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.7 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 60.00 \text{ in}$$

$$J = 0.427$$

$$135.981$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.6$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.7 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.848 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &21.4323 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.32 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 20° 110mph 30psf 5ft 7-05.r3dPage 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	271.049	1	.078	2	.261	1	0	10	0	4	0	15
30			min	-355.059	3	.014	15	-.442	5	0	4	0	3	0	6
31		16	max	271.155	1	.046	2	.261	1	0	10	0	1	0	15
32			min	-354.979	3	-.004	3	-.538	5	0	4	0	3	0	6
33		17	max	271.262	1	.014	2	.261	1	0	10	0	1	0	15
34			min	-354.899	3	-.028	3	-.635	5	0	4	0	3	0	6
35		18	max	271.368	1	-.015	15	.261	1	0	10	0	1	0	15
36			min	-354.819	3	-.059	4	-.731	5	0	4	0	3	0	6
37		19	max	271.475	1	-.025	15	.261	1	0	10	0	1	0	15
38			min	-354.739	3	-.1	4	-.828	5	0	4	0	3	0	6
39	M3	1	max	87.381	2	1.795	6	-.011	10	0	5	0	1	0	6
40			min	-84.227	3	.421	15	-1.382	4	0	1	0	10	0	15
41		2	max	87.314	2	1.617	6	-.011	10	0	5	0	1	0	6
42			min	-84.277	3	.379	15	-1.248	4	0	1	0	10	0	15
43		3	max	87.246	2	1.439	6	-.011	10	0	5	0	1	0	2
44			min	-84.328	3	.337	15	-1.114	4	0	1	0	10	0	3
45		4	max	87.178	2	1.262	6	-.011	10	0	5	0	1	0	15
46			min	-84.379	3	.295	15	-.981	4	0	1	0	5	0	4
47		5	max	87.11	2	1.084	6	-.011	10	0	5	0	1	0	15
48			min	-84.43	3	.254	15	-.847	4	0	1	0	5	0	4
49		6	max	87.042	2	.906	6	-.011	10	0	5	0	1	0	15
50			min	-84.481	3	.212	15	-.714	4	0	1	0	5	0	4
51		7	max	86.974	2	.729	6	-.011	10	0	5	0	1	0	15
52			min	-84.532	3	.17	15	-.58	4	0	1	0	5	0	4
53		8	max	86.906	2	.551	6	-.011	10	0	5	0	1	0	15
54			min	-84.583	3	.128	15	-.446	4	0	1	0	5	0	4
55		9	max	86.839	2	.373	6	-.011	10	0	5	0	1	0	15
56			min	-84.634	3	.087	15	-.313	4	0	1	0	5	-.001	4
57		10	max	86.771	2	.196	6	-.011	10	0	5	0	1	0	15
58			min	-84.685	3	.045	15	-.247	1	0	1	0	5	-.001	4
59		11	max	86.703	2	.034	2	.006	5	0	5	0	1	0	15
60			min	-84.736	3	-.002	3	-.247	1	0	1	0	5	-.001	4
61		12	max	86.635	2	-.039	15	.14	5	0	5	0	1	0	15
62			min	-84.786	3	-.16	4	-.247	1	0	1	0	5	-.001	4
63		13	max	86.567	2	-.08	15	.274	5	0	5	0	1	0	15
64			min	-84.837	3	-.337	4	-.247	1	0	1	0	5	-.001	4
65		14	max	86.499	2	-.122	15	.407	5	0	5	0	1	0	15
66			min	-84.888	3	-.515	4	-.247	1	0	1	0	5	-.001	4
67		15	max	86.431	2	-.164	15	.541	5	0	5	0	9	0	15
68			min	-84.939	3	-.693	4	-.247	1	0	1	0	5	0	4
69		16	max	86.364	2	-.206	15	.674	5	0	5	0	10	0	15
70			min	-84.99	3	-.87	4	-.247	1	0	1	0	4	0	4
71		17	max	86.296	2	-.247	15	.808	5	0	5	0	10	0	15
72			min	-85.041	3	-1.048	4	-.247	1	0	1	0	4	0	4
73		18	max	86.228	2	-.289	15	.942	5	0	5	0	10	0	15
74			min	-85.092	3	-1.226	4	-.247	1	0	1	0	4	0	4
75		19	max	86.16	2	-.331	15	1.075	5	0	5	0	5	0	1
76			min	-85.143	3	-1.403	4	-.247	1	0	1	0	1	0	1
77	M4	1	max	374.902	1	0	1	-.036	10	0	1	0	5	0	1
78			min	-73.652	3	0	1	-18.975	4	0	1	0	2	0	1
79		2	max	374.967	1	0	1	-.036	10	0	1	0	12	0	1
80			min	-73.603	3	0	1	-19.032	4	0	1	-.002	4	0	1
81		3	max	375.031	1	0	1	-.036	10	0	1	0	10	0	1
82			min	-73.555	3	0	1	-19.088	4	0	1	-.003	4	0	1
83		4	max	375.096	1	0	1	-.036	10	0	1	0	10	0	1
84			min	-73.506	3	0	1	-19.144	4	0	1	-.005	4	0	1
85		5	max	375.161	1	0	1	-.036	10	0	1	0	10	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86		min	-73.457	3	0	1	-19.2	4	0	1	-.007	4	0	1
87	6	max	375.225	1	0	1	-.036	10	0	1	0	10	0	1
88		min	-73.409	3	0	1	-19.256	4	0	1	-.009	4	0	1
89	7	max	375.29	1	0	1	-.036	10	0	1	0	10	0	1
90		min	-73.36	3	0	1	-19.312	4	0	1	-.01	4	0	1
91	8	max	375.355	1	0	1	-.036	10	0	1	0	10	0	1
92		min	-73.312	3	0	1	-19.368	4	0	1	-.012	4	0	1
93	9	max	375.42	1	0	1	-.036	10	0	1	0	10	0	1
94		min	-73.263	3	0	1	-19.424	4	0	1	-.014	4	0	1
95	10	max	375.484	1	0	1	-.036	10	0	1	0	10	0	1
96		min	-73.215	3	0	1	-19.48	4	0	1	-.015	4	0	1
97	11	max	375.549	1	0	1	-.036	10	0	1	0	10	0	1
98		min	-73.166	3	0	1	-19.536	4	0	1	-.017	4	0	1
99	12	max	375.614	1	0	1	-.036	10	0	1	0	10	0	1
100		min	-73.118	3	0	1	-19.592	4	0	1	-.019	4	0	1
101	13	max	375.678	1	0	1	-.036	10	0	1	0	10	0	1
102		min	-73.069	3	0	1	-19.648	4	0	1	-.021	4	0	1
103	14	max	375.743	1	0	1	-.036	10	0	1	0	10	0	1
104		min	-73.021	3	0	1	-19.705	4	0	1	-.022	4	0	1
105	15	max	375.808	1	0	1	-.036	10	0	1	0	10	0	1
106		min	-72.972	3	0	1	-19.761	4	0	1	-.024	4	0	1
107	16	max	375.872	1	0	1	-.036	10	0	1	0	10	0	1
108		min	-72.924	3	0	1	-19.817	4	0	1	-.026	4	0	1
109	17	max	375.937	1	0	1	-.036	10	0	1	0	10	0	1
110		min	-72.875	3	0	1	-19.873	4	0	1	-.028	4	0	1
111	18	max	376.002	1	0	1	-.036	10	0	1	0	10	0	1
112		min	-72.827	3	0	1	-19.929	4	0	1	-.03	4	0	1
113	19	max	376.067	1	0	1	-.036	10	0	1	0	10	0	1
114		min	-72.778	3	0	1	-19.985	4	0	1	-.031	4	0	1
115	M6	1	max	854.183	1	.631	.94	4	0	3	0	3	0	1
116		min	-1115.49	3	.144	15	-.206	3	0	5	0	2	0	1
117	2	max	854.29	1	.589	6	.843	4	0	3	0	4	0	15
118		min	-1115.41	3	.134	15	-.206	3	0	5	0	2	0	6
119	3	max	854.397	1	.548	6	.747	4	0	3	0	4	0	15
120		min	-1115.33	3	.125	15	-.206	3	0	5	0	2	0	6
121	4	max	854.503	1	.507	6	.65	4	0	3	0	4	0	15
122		min	-1115.25	3	.115	15	-.206	3	0	5	0	2	0	6
123	5	max	854.61	1	.466	6	.554	4	0	3	0	4	0	15
124		min	-1115.17	3	.105	15	-.206	3	0	5	0	3	0	6
125	6	max	854.716	1	.424	6	.457	4	0	3	0	4	0	15
126		min	-1115.091	3	.096	15	-.206	3	0	5	0	3	0	6
127	7	max	854.823	1	.388	2	.361	4	0	3	0	4	0	15
128		min	-1115.011	3	.086	15	-.206	3	0	5	0	3	0	6
129	8	max	854.929	1	.356	2	.264	4	0	3	0	4	0	15
130		min	-1114.931	3	.076	15	-.206	3	0	5	0	3	0	6
131	9	max	855.036	1	.324	2	.168	4	0	3	0	4	0	15
132		min	-1114.851	3	.066	15	-.206	3	0	5	0	3	0	6
133	10	max	855.142	1	.292	2	.082	14	0	3	0	4	0	15
134		min	-1114.771	3	.057	15	-.206	3	0	5	0	3	0	6
135	11	max	855.249	1	.26	2	.08	1	0	3	0	4	0	15
136		min	-1114.691	3	.047	15	-.206	3	0	5	0	3	0	6
137	12	max	855.355	1	.227	2	.08	1	0	3	0	4	0	15
138		min	-1114.611	3	.037	15	-.206	3	0	5	0	3	0	2
139	13	max	855.462	1	.195	2	.08	1	0	3	0	4	0	15
140		min	-1114.531	3	.022	12	-.243	5	0	5	0	3	0	2
141	14	max	855.569	1	.163	2	.08	1	0	3	0	4	0	15
142		min	-1114.451	3	.005	3	-.34	5	0	5	0	3	0	2



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	855.675	1	.131	2	.08	1	0	3	0	4	0	15
144			min	-1114.371	3	-.019	3	-.436	5	0	5	0	3	0	2
145		16	max	855.782	1	.099	2	.08	1	0	3	0	4	0	15
146			min	-1114.291	3	-.043	3	-.533	5	0	5	0	3	0	2
147		17	max	855.888	1	.067	2	.08	1	0	3	0	4	0	15
148			min	-1114.212	3	-.068	3	-.629	5	0	5	0	3	0	2
149		18	max	855.995	1	.034	2	.08	1	0	3	0	4	0	15
150			min	-1114.132	3	-.092	3	-.725	5	0	5	0	3	0	2
151		19	max	856.101	1	.002	2	.08	1	0	3	0	14	0	15
152			min	-1114.052	3	-.116	3	-.822	5	0	5	0	3	0	2
153	M7	1	max	320.131	2	1.805	4	.008	3	0	1	0	4	0	2
154			min	-239.462	3	.428	15	-1.412	4	0	3	0	3	0	12
155		2	max	320.064	2	1.627	4	.008	3	0	1	0	4	0	2
156			min	-239.513	3	.387	15	-1.278	4	0	3	0	3	0	12
157		3	max	319.996	2	1.449	4	.008	3	0	1	0	4	0	2
158			min	-239.564	3	.345	15	-1.145	4	0	3	0	3	0	3
159		4	max	319.928	2	1.272	4	.008	3	0	1	0	1	0	2
160			min	-239.615	3	.303	15	-1.011	4	0	3	0	3	0	3
161		5	max	319.86	2	1.094	4	.008	3	0	1	0	1	0	15
162			min	-239.666	3	.261	15	-.877	4	0	3	0	5	0	6
163		6	max	319.792	2	.916	4	.008	3	0	1	0	1	0	15
164			min	-239.717	3	.22	15	-.744	4	0	3	0	5	0	6
165		7	max	319.724	2	.739	4	.008	3	0	1	0	1	0	15
166			min	-239.768	3	.178	15	-.61	4	0	3	0	5	0	6
167		8	max	319.656	2	.561	4	.008	3	0	1	0	1	0	15
168			min	-239.818	3	.136	15	-.476	4	0	3	0	5	0	6
169		9	max	319.589	2	.383	4	.008	3	0	1	0	1	0	15
170			min	-239.869	3	.094	15	-.343	4	0	3	0	5	-.001	6
171		10	max	319.521	2	.211	2	.008	3	0	1	0	1	0	15
172			min	-239.92	3	.046	12	-.209	4	0	3	0	5	-.001	6
173		11	max	319.453	2	.072	2	.008	3	0	1	0	1	0	15
174			min	-239.971	3	-.039	3	-.076	4	0	3	0	5	-.001	6
175		12	max	319.385	2	-.031	15	.059	5	0	1	0	1	0	15
176			min	-240.022	3	-.15	6	-.008	11	0	3	0	5	-.001	6
177		13	max	319.317	2	-.073	15	.193	5	0	1	0	1	0	15
178			min	-240.073	3	-.327	6	-.008	11	0	3	0	5	-.001	6
179		14	max	319.249	2	-.115	15	.326	5	0	1	0	1	0	15
180			min	-240.124	3	-.505	6	-.008	11	0	3	0	5	-.001	6
181		15	max	319.181	2	-.156	15	.46	5	0	1	0	1	0	15
182			min	-240.175	3	-.683	6	-.008	11	0	3	0	5	0	6
183		16	max	319.114	2	-.198	15	.594	5	0	1	0	1	0	15
184			min	-240.226	3	-.86	6	-.008	11	0	3	0	5	0	6
185		17	max	319.046	2	-.24	15	.727	5	0	1	0	1	0	15
186			min	-240.276	3	-1.038	6	-.008	11	0	3	0	5	0	6
187		18	max	318.978	2	-.282	15	.861	5	0	1	0	1	0	15
188			min	-240.327	3	-1.216	6	-.008	11	0	3	0	5	0	6
189		19	max	318.91	2	-.323	15	.995	5	0	1	0	1	0	1
190			min	-240.378	3	-1.393	6	-.008	11	0	3	0	3	0	1
191	M8	1	max	1086.401	1	0	1	.325	1	0	1	0	4	0	1
192			min	-263.079	3	0	1	-19.325	4	0	1	0	1	0	1
193		2	max	1086.466	1	0	1	.325	1	0	1	0	1	0	1
194			min	-263.031	3	0	1	-19.381	4	0	1	-.002	4	0	1
195		3	max	1086.53	1	0	1	.325	1	0	1	0	1	0	1
196			min	-262.982	3	0	1	-19.437	4	0	1	-.003	4	0	1
197		4	max	1086.595	1	0	1	.325	1	0	1	0	1	0	1
198			min	-262.934	3	0	1	-19.493	4	0	1	-.005	4	0	1
199		5	max	1086.66	1	0	1	.325	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-262.885	3	0	1	-19.549	4	0	1	-.007	4	0	1
201		6	max	1086.724	1	0	1	.325	1	0	1	0	1	0	1
202			min	-262.837	3	0	1	-19.605	4	0	1	-.009	4	0	1
203		7	max	1086.789	1	0	1	.325	1	0	1	0	1	0	1
204			min	-262.788	3	0	1	-19.661	4	0	1	-.01	4	0	1
205		8	max	1086.854	1	0	1	.325	1	0	1	0	1	0	1
206			min	-262.74	3	0	1	-19.717	4	0	1	-.012	4	0	1
207		9	max	1086.919	1	0	1	.325	1	0	1	0	1	0	1
208			min	-262.691	3	0	1	-19.773	4	0	1	-.014	4	0	1
209		10	max	1086.983	1	0	1	.325	1	0	1	0	1	0	1
210			min	-262.643	3	0	1	-19.83	4	0	1	-.016	4	0	1
211		11	max	1087.048	1	0	1	.325	1	0	1	0	1	0	1
212			min	-262.594	3	0	1	-19.886	4	0	1	-.018	4	0	1
213		12	max	1087.113	1	0	1	.325	1	0	1	0	1	0	1
214			min	-262.546	3	0	1	-19.942	4	0	1	-.019	4	0	1
215		13	max	1087.177	1	0	1	.325	1	0	1	0	1	0	1
216			min	-262.497	3	0	1	-19.998	4	0	1	-.021	4	0	1
217		14	max	1087.242	1	0	1	.325	1	0	1	0	1	0	1
218			min	-262.449	3	0	1	-20.054	4	0	1	-.023	4	0	1
219		15	max	1087.307	1	0	1	.325	1	0	1	0	1	0	1
220			min	-262.4	3	0	1	-20.11	4	0	1	-.025	4	0	1
221		16	max	1087.372	1	0	1	.325	1	0	1	0	1	0	1
222			min	-262.351	3	0	1	-20.166	4	0	1	-.026	4	0	1
223		17	max	1087.436	1	0	1	.325	1	0	1	0	1	0	1
224			min	-262.303	3	0	1	-20.222	4	0	1	-.028	4	0	1
225		18	max	1087.501	1	0	1	.325	1	0	1	0	1	0	1
226			min	-262.254	3	0	1	-20.278	4	0	1	-.03	4	0	1
227		19	max	1087.566	1	0	1	.325	1	0	1	0	1	0	1
228			min	-262.206	3	0	1	-20.334	4	0	1	-.032	4	0	1
229	M10	1	max	271.572	1	.671	4	1.08	5	0	1	0	1	0	1
230			min	-325.427	3	.169	15	-.103	1	-.001	5	0	3	0	1
231		2	max	271.678	1	.63	4	.983	5	0	1	0	4	0	15
232			min	-325.347	3	.159	15	-.103	1	-.001	5	0	3	0	4
233		3	max	271.785	1	.589	4	.887	5	0	1	0	4	0	15
234			min	-325.267	3	.149	15	-.103	1	-.001	5	0	3	0	4
235		4	max	271.892	1	.548	4	.791	5	0	1	0	4	0	15
236			min	-325.188	3	.14	15	-.103	1	-.001	5	0	3	0	4
237		5	max	271.998	1	.506	4	.694	5	0	1	0	4	0	15
238			min	-325.108	3	.13	15	-.103	1	-.001	5	0	3	0	4
239		6	max	272.105	1	.465	4	.598	5	0	1	0	4	0	15
240			min	-325.028	3	.12	15	-.103	1	-.001	5	0	3	0	4
241		7	max	272.211	1	.424	4	.501	5	0	1	0	4	0	15
242			min	-324.948	3	.111	15	-.103	1	-.001	5	0	3	0	4
243		8	max	272.318	1	.382	4	.405	5	0	1	0	4	0	15
244			min	-324.868	3	.101	15	-.103	1	-.001	5	0	3	0	4
245		9	max	272.424	1	.341	4	.308	5	0	1	0	5	0	15
246			min	-324.788	3	.091	15	-.103	1	-.001	5	0	3	0	4
247		10	max	272.531	1	.3	4	.212	5	0	1	0	5	0	15
248			min	-324.708	3	.082	15	-.103	1	-.001	5	0	3	0	4
249		11	max	272.637	1	.259	4	.115	5	0	1	0	5	0	15
250			min	-324.628	3	.072	15	-.103	1	-.001	5	0	3	0	4
251		12	max	272.744	1	.217	4	.019	5	0	1	0	5	0	15
252			min	-324.548	3	.062	15	-.103	1	-.001	5	0	3	0	4
253		13	max	272.85	1	.176	4	-.009	10	0	1	0	5	0	15
254			min	-324.468	3	.052	15	-.103	1	-.001	5	0	3	0	4
255		14	max	272.957	1	.135	4	-.009	10	0	1	0	5	0	15
256			min	-324.389	3	.043	15	-.185	4	-.001	5	0	3	0	4



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	273.064	1	.094	4	-.009	10	0	1	0	5	0	15
258		min	-324.309	3	.033	15	-.282	4	-.001	5	0	3	0	4
259	16	max	273.17	1	.052	4	-.009	10	0	1	0	5	0	15
260		min	-324.229	3	.007	9	-.378	4	-.001	5	0	3	0	4
261	17	max	273.277	1	.02	5	-.009	10	0	1	0	5	0	15
262		min	-324.149	3	-.02	9	-.475	4	-.001	5	0	3	0	4
263	18	max	273.383	1	.005	5	-.009	10	0	1	0	5	0	15
264		min	-324.069	3	-.047	1	-.571	4	-.001	5	0	3	0	4
265	19	max	273.49	1	-.006	15	-.009	10	0	1	0	5	0	15
266		min	-323.989	3	-.079	1	-.668	4	-.001	5	0	3	0	4
267	M11	1	max	86.907	2	1.792	.273	1	0	4	.001	5	0	6
268		min	-84.838	3	.419	15	-1.251	5	0	10	0	1	0	15
269	2	max	86.839	2	1.614	6	.273	1	0	4	0	5	0	6
270		min	-84.889	3	.377	15	-1.117	5	0	10	0	1	0	15
271	3	max	86.771	2	1.436	6	.273	1	0	4	0	5	0	2
272		min	-84.939	3	.335	15	-.983	5	0	10	0	1	0	3
273	4	max	86.703	2	1.259	6	.273	1	0	4	0	5	0	15
274		min	-84.99	3	.293	15	-.85	5	0	10	0	1	0	4
275	5	max	86.635	2	1.081	6	.273	1	0	4	0	3	0	15
276		min	-85.041	3	.252	15	-.716	5	0	10	0	1	0	4
277	6	max	86.567	2	.903	6	.273	1	0	4	0	3	0	15
278		min	-85.092	3	.21	15	-.583	5	0	10	0	1	0	4
279	7	max	86.499	2	.726	6	.273	1	0	4	0	3	0	15
280		min	-85.143	3	.168	15	-.449	5	0	10	0	1	0	4
281	8	max	86.432	2	.548	6	.273	1	0	4	0	3	0	15
282		min	-85.194	3	.126	15	-.315	5	0	10	0	1	-.001	4
283	9	max	86.364	2	.37	6	.273	1	0	4	0	3	0	15
284		min	-85.245	3	.084	15	-.182	5	0	10	0	1	-.001	4
285	10	max	86.296	2	.193	6	.273	1	0	4	0	3	0	15
286		min	-85.296	3	.043	15	-.048	5	0	10	0	4	-.001	4
287	11	max	86.228	2	.034	2	.273	1	0	4	0	3	0	15
288		min	-85.347	3	-.019	3	-.017	3	0	10	0	4	-.001	4
289	12	max	86.16	2	-.041	15	.276	4	0	4	0	3	0	15
290		min	-85.397	3	-.163	4	-.017	3	0	10	0	4	-.001	4
291	13	max	86.092	2	-.083	15	.41	4	0	4	0	3	0	15
292		min	-85.448	3	-.341	4	-.017	3	0	10	0	4	-.001	4
293	14	max	86.024	2	-.124	15	.543	4	0	4	0	3	0	15
294		min	-85.499	3	-.518	4	-.017	3	0	10	0	5	-.001	4
295	15	max	85.956	2	-.166	15	.677	4	0	4	0	3	0	15
296		min	-85.55	3	-.696	4	-.017	3	0	10	0	10	0	4
297	16	max	85.889	2	-.208	15	.811	4	0	4	0	3	0	15
298		min	-85.601	3	-.873	4	-.017	3	0	10	0	10	0	4
299	17	max	85.821	2	-.25	15	.944	4	0	4	0	4	0	15
300		min	-85.652	3	-1.051	4	-.017	3	0	10	0	10	0	4
301	18	max	85.753	2	-.291	15	1.078	4	0	4	0	4	0	15
302		min	-85.703	3	-1.229	4	-.017	3	0	10	0	10	0	4
303	19	max	85.685	2	-.333	15	1.212	4	0	4	0	4	0	1
304		min	-85.754	3	-1.406	4	-.017	3	0	10	0	10	0	1
305	M12	1	max	374.899	1	0	1.282	1	0	1	0	4	0	1
306		min	-73.241	3	0	1	-17.727	5	0	1	0	3	0	1
307	2	max	374.964	1	0	1	1.282	1	0	1	0	1	0	1
308		min	-73.192	3	0	1	-17.783	5	0	1	-.002	5	0	1
309	3	max	375.028	1	0	1	1.282	1	0	1	0	1	0	1
310		min	-73.144	3	0	1	-17.839	5	0	1	-.003	5	0	1
311	4	max	375.093	1	0	1	1.282	1	0	1	0	1	0	1
312		min	-73.095	3	0	1	-17.895	5	0	1	-.005	5	0	1
313	5	max	375.158	1	0	1	1.282	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-73.047	3	0	1	-17.951	5	0	1	-.006	5	0	1
315	6	max	375.223	1	0	1	1.282	1	0	1	0	1	0	1
316		min	-72.998	3	0	1	-18.008	5	0	1	-.008	5	0	1
317	7	max	375.287	1	0	1	1.282	1	0	1	0	1	0	1
318		min	-72.95	3	0	1	-18.064	5	0	1	-.01	5	0	1
319	8	max	375.352	1	0	1	1.282	1	0	1	0	1	0	1
320		min	-72.901	3	0	1	-18.12	5	0	1	-.011	5	0	1
321	9	max	375.417	1	0	1	1.282	1	0	1	0	1	0	1
322		min	-72.853	3	0	1	-18.176	5	0	1	-.013	5	0	1
323	10	max	375.481	1	0	1	1.282	1	0	1	.001	1	0	1
324		min	-72.804	3	0	1	-18.232	5	0	1	-.014	5	0	1
325	11	max	375.546	1	0	1	1.282	1	0	1	.001	1	0	1
326		min	-72.756	3	0	1	-18.288	5	0	1	-.016	5	0	1
327	12	max	375.611	1	0	1	1.282	1	0	1	.001	1	0	1
328		min	-72.707	3	0	1	-18.344	5	0	1	-.018	5	0	1
329	13	max	375.675	1	0	1	1.282	1	0	1	.001	1	0	1
330		min	-72.659	3	0	1	-18.4	5	0	1	-.019	5	0	1
331	14	max	375.74	1	0	1	1.282	1	0	1	.002	1	0	1
332		min	-72.61	3	0	1	-18.456	5	0	1	-.021	5	0	1
333	15	max	375.805	1	0	1	1.282	1	0	1	.002	1	0	1
334		min	-72.562	3	0	1	-18.512	5	0	1	-.023	5	0	1
335	16	max	375.87	1	0	1	1.282	1	0	1	.002	1	0	1
336		min	-72.513	3	0	1	-18.568	5	0	1	-.024	5	0	1
337	17	max	375.934	1	0	1	1.282	1	0	1	.002	1	0	1
338		min	-72.465	3	0	1	-18.624	5	0	1	-.026	5	0	1
339	18	max	375.999	1	0	1	1.282	1	0	1	.002	1	0	1
340		min	-72.416	3	0	1	-18.68	5	0	1	-.028	5	0	1
341	19	max	376.064	1	0	1	1.282	1	0	1	.002	1	0	1
342		min	-72.367	3	0	1	-18.737	5	0	1	-.029	5	0	1
343	M1	1	max	81.119	1	335.253	3	-1.128	10	0	.051	1	.014	1
344		min	4.5	12	-272.61	1	-25.908	1	0	3	.002	10	-.015	3
345	2	max	81.214	1	335.057	3	-1.128	10	0	1	.045	1	.073	1
346		min	4.548	12	-272.872	1	-25.908	1	0	3	.002	10	-.087	3
347	3	max	66.7	1	5.115	14	-1.119	10	0	5	.039	1	.132	1
348		min	.622	10	-20.765	3	-25.751	1	0	1	.002	10	-.159	3
349	4	max	66.795	1	4.857	14	-1.119	10	0	5	.034	1	.132	1
350		min	.702	10	-20.962	3	-25.751	1	0	1	.001	10	-.154	3
351	5	max	66.891	1	4.599	14	-1.119	10	0	5	.028	1	.133	1
352		min	.781	10	-21.159	3	-25.751	1	0	1	.001	10	-.15	3
353	6	max	66.986	1	4.341	14	-1.119	10	0	5	.022	1	.134	1
354		min	.861	10	-21.356	3	-25.751	1	0	1	0	10	-.145	3
355	7	max	67.082	1	4.083	14	-1.119	10	0	5	.017	1	.135	1
356		min	.941	10	-21.552	3	-25.751	1	0	1	0	10	-.14	3
357	8	max	67.177	1	3.826	14	-1.119	10	0	5	.011	1	.139	2
358		min	1.02	10	-21.749	3	-25.751	1	0	1	0	10	-.136	3
359	9	max	67.273	1	3.601	9	-1.119	10	0	5	.006	1	.143	2
360		min	1.1	10	-21.946	3	-25.751	1	0	1	0	10	-.131	3
361	10	max	67.368	1	3.383	9	-1.119	10	0	5	.001	3	.147	2
362		min	1.179	10	-22.143	3	-25.751	1	0	1	0	10	-.126	3
363	11	max	67.464	1	3.164	9	-1.119	10	0	5	0	3	.151	2
364		min	1.259	10	-22.34	3	-25.751	1	0	1	-.005	1	-.121	3
365	12	max	67.559	1	2.945	9	-1.119	10	0	5	0	12	.155	2
366		min	1.339	10	-22.536	3	-25.751	1	0	1	-.011	1	-.116	3
367	13	max	67.655	1	2.727	9	-1.119	10	0	5	0	12	.16	2
368		min	1.418	10	-22.733	3	-25.751	1	0	1	-.017	1	-.111	3
369	14	max	67.75	1	2.508	9	-1.119	10	0	5	0	10	.164	2
370		min	1.498	10	-22.93	3	-25.751	1	0	1	-.022	1	-.107	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	67.846	1	2.289	9	-1.119	10	0	5	-.001	10	.169	2
372		min	1.577	10	-23.127	3	-25.751	1	0	1	-.028	1	-.102	3
373	16	max	79.667	2	40.711	2	-1.13	10	0	1	-.001	10	.173	2
374		min	-30.226	3	-85.183	3	-25.974	1	0	5	-.034	1	-.096	3
375	17	max	79.762	2	40.448	2	-1.13	10	0	1	-.002	10	.164	2
376		min	-30.154	3	-85.38	3	-25.974	1	0	5	-.039	1	-.077	3
377	18	max	-3.244	12	341.384	2	-1.166	10	0	5	-.002	10	.091	2
378		min	-81.187	1	-154.861	3	-32.791	4	0	2	-.045	1	-.044	3
379	19	max	-3.196	12	341.121	2	-1.166	10	0	5	-.002	10	.017	2
380		min	-81.092	1	-155.057	3	-32.549	4	0	2	-.051	1	-.011	3
381	M5	1	max	191.714	1	1079.328	3	0	2	0	.036	4	.03	3
382		min	1.973	15	-874.204	1	-48.631	3	0	5	0	10	-.028	1
383	2	max	191.81	1	1079.131	3	0	2	0	1	.031	4	.161	1
384		min	2.002	15	-874.466	1	-48.631	3	0	5	-.003	3	-.205	3
385	3	max	144.798	1	6.605	9	5.244	3	0	3	.026	4	.347	1
386		min	.456	10	-65.869	3	-19.859	4	0	4	-.014	3	-.434	3
387	4	max	144.893	1	6.387	9	5.244	3	0	3	.022	4	.352	1
388		min	.535	10	-66.066	3	-19.617	4	0	4	-.012	3	-.419	3
389	5	max	144.989	1	6.168	9	5.244	3	0	3	.017	4	.357	1
390		min	.615	10	-66.263	3	-19.375	4	0	4	-.011	3	-.405	3
391	6	max	145.084	1	5.949	9	5.244	3	0	3	.013	4	.362	1
392		min	.694	10	-66.46	3	-19.133	4	0	4	-.01	3	-.391	3
393	7	max	145.18	1	5.731	9	5.244	3	0	3	.009	4	.367	1
394		min	.774	10	-66.657	3	-18.891	4	0	4	-.009	3	-.376	3
395	8	max	145.275	1	5.512	9	5.244	3	0	3	.005	4	.372	2
396		min	.853	10	-66.853	3	-18.649	4	0	4	-.008	3	-.362	3
397	9	max	145.371	1	5.293	9	5.244	3	0	3	.001	4	.384	2
398		min	.933	10	-67.05	3	-18.407	4	0	4	-.007	3	-.347	3
399	10	max	145.466	1	5.075	9	5.244	3	0	3	0	2	.397	2
400		min	1.013	10	-67.247	3	-18.165	4	0	4	-.006	3	-.333	3
401	11	max	145.562	1	4.856	9	5.244	3	0	3	0	2	.41	2
402		min	1.092	10	-67.444	3	-17.923	4	0	4	-.007	4	-.318	3
403	12	max	145.657	1	4.637	9	5.244	3	0	3	0	2	.423	2
404		min	1.172	10	-67.641	3	-17.681	4	0	4	-.011	4	-.303	3
405	13	max	145.753	1	4.419	9	5.244	3	0	3	0	2	.436	2
406		min	1.251	10	-67.837	3	-17.439	4	0	4	-.014	4	-.289	3
407	14	max	145.848	1	4.2	9	5.244	3	0	3	0	2	.449	2
408		min	1.331	10	-68.034	3	-17.197	4	0	4	-.018	4	-.274	3
409	15	max	145.944	1	3.981	9	5.244	3	0	3	0	3	.463	2
410		min	1.411	10	-68.231	3	-16.955	4	0	4	-.022	4	-.259	3
411	16	max	260.695	2	170.747	2	5.22	3	0	3	0	3	.474	2
412		min	-96.902	3	-241.585	3	-15.725	4	0	4	-.026	4	-.243	3
413	17	max	260.79	2	170.485	2	5.22	3	0	3	.002	3	.437	2
414		min	-96.83	3	-241.781	3	-15.483	4	0	4	-.029	4	-.19	3
415	18	max	-4.766	12	1093.023	2	4.824	3	0	4	.003	3	.203	2
416		min	-191.866	1	-489.888	3	-34.082	5	0	1	-.036	4	-.085	3
417	19	max	-4.719	12	1092.76	2	4.824	3	0	4	.004	3	.021	3
418		min	-191.771	1	-490.085	3	-33.84	5	0	1	-.044	4	-.034	2
419	M9	1	max	80.888	1	335.218	3	141.821	4	0	3	0	.014	1
420		min	.775	15	-272.608	1	1.128	10	0	1	-.05	1	-.015	3
421	2	max	80.984	1	335.021	3	142.063	4	0	3	.029	5	.073	1
422		min	.804	15	-272.871	1	1.128	10	0	1	-.045	1	-.087	3
423	3	max	66.884	1	4.892	9	25.179	1	0	1	.057	5	.131	1
424		min	.754	15	-20.697	3	-25.563	5	0	10	-.038	1	-.159	3
425	4	max	66.979	1	4.673	9	25.179	1	0	1	.051	5	.132	1
426		min	.783	15	-20.894	3	-25.321	5	0	10	-.033	1	-.154	3
427	5	max	67.075	1	4.455	9	25.179	1	0	1	.046	5	.133	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	.811	15	-21.091	3	-25.079	5	0	10	-.027	1	-.149	3
429		6	max	67.17	1	4.236	9	25.179	1	0	1	.04	5	.134	1
430			min	.84	15	-21.288	3	-24.837	5	0	10	-.022	1	-.145	3
431		7	max	67.266	1	4.017	9	25.179	1	0	1	.035	5	.135	1
432			min	.869	15	-21.485	3	-24.595	5	0	10	-.016	1	-.14	3
433		8	max	67.361	1	3.799	9	25.179	1	0	1	.03	5	.139	2
434			min	.898	15	-21.681	3	-24.353	5	0	10	-.011	1	-.136	3
435		9	max	67.457	1	3.58	9	25.179	1	0	1	.024	5	.143	2
436			min	.927	15	-21.878	3	-24.111	5	0	10	-.006	1	-.131	3
437		10	max	67.552	1	3.361	9	25.179	1	0	1	.019	4	.147	2
438			min	.955	15	-22.075	3	-23.869	5	0	10	0	1	-.126	3
439		11	max	67.648	1	3.143	9	25.179	1	0	1	.015	4	.151	2
440			min	.984	15	-22.272	3	-23.627	5	0	10	0	10	-.121	3
441		12	max	67.743	1	2.924	9	25.179	1	0	1	.011	4	.155	2
442			min	1.013	15	-22.469	3	-23.385	5	0	10	0	10	-.116	3
443		13	max	67.839	1	2.705	9	25.179	1	0	1	.016	1	.16	2
444			min	1.042	15	-22.665	3	-23.143	5	0	10	0	10	-.112	3
445		14	max	67.934	1	2.487	9	25.179	1	0	1	.022	1	.164	2
446			min	1.071	15	-22.862	3	-22.901	5	0	10	-.001	5	-.107	3
447		15	max	68.03	1	2.268	9	25.179	1	0	1	.027	1	.169	2
448			min	1.1	15	-23.059	3	-22.659	5	0	10	-.006	5	-.102	3
449		16	max	79.782	2	40.394	2	25.43	1	0	10	.033	1	.172	2
450			min	-30.713	3	-85.565	3	-21.265	5	0	4	-.01	5	-.096	3
451		17	max	79.878	2	40.131	2	25.43	1	0	10	.039	1	.164	2
452			min	-30.642	3	-85.762	3	-21.023	5	0	4	-.014	5	-.077	3
453		18	max	7.294	5	341.384	2	26.698	1	0	2	.044	1	.091	2
454			min	-80.954	1	-154.856	3	-38.489	5	0	3	-.023	5	-.044	3
455		19	max	7.339	5	341.121	2	26.698	1	0	2	.05	1	.017	2
456			min	-80.859	1	-155.053	3	-38.247	5	0	3	-.031	5	-.011	3
457	M13	1	max	141.821	4	272.322	1	-.775	15	.014	1	.05	1	0	1
458			min	1.128	10	-335.228	3	-80.884	1	-.015	3	0	15	0	3
459		2	max	136.32	4	193.607	1	-.12	15	.014	1	.011	1	.159	3
460			min	1.128	10	-238.022	3	-61.233	1	-.015	3	0	10	-.129	1
461		3	max	130.819	4	114.891	1	.713	5	.014	1	.006	3	.264	3
462			min	1.128	10	-140.816	3	-41.582	1	-.015	3	-.018	1	-.215	1
463		4	max	125.318	4	36.176	1	1.726	5	.014	1	.004	3	.316	3
464			min	1.128	10	-43.611	3	-21.93	1	-.015	3	-.035	1	-.257	1
465		5	max	119.817	4	53.595	3	2.739	5	.014	1	.002	5	.313	3
466			min	1.128	10	-42.539	1	-2.636	3	-.015	3	-.042	1	-.255	1
467		6	max	114.316	4	150.801	3	17.372	1	.014	1	.004	5	.256	3
468			min	1.128	10	-121.255	1	-1.678	3	-.015	3	-.038	1	-.21	1
469		7	max	108.815	4	248.007	3	37.023	1	.014	1	.006	5	.145	3
470			min	1.128	10	-199.97	1	-.72	3	-.015	3	-.023	1	-.121	1
471		8	max	103.315	4	345.213	3	56.674	1	.014	1	.01	4	.012	1
472			min	1.128	10	-278.685	1	.239	3	-.015	3	0	12	-.019	3
473		9	max	97.814	4	442.419	3	76.326	1	.014	1	.04	1	.189	1
474			min	1.128	10	-357.4	1	.901	12	-.015	3	0	12	-.238	3
475		10	max	92.313	4	539.625	3	95.977	1	.014	1	.088	1	.409	1
476			min	1.128	10	-436.116	1	1.54	12	-.015	3	-.019	5	-.511	3
477		11	max	64.193	4	357.4	1	5.5	5	.015	3	.04	1	.189	1
478			min	1.128	10	-442.419	3	-76.095	1	-.014	1	-.016	5	-.238	3
479		12	max	58.692	4	278.685	1	6.513	5	.015	3	.004	2	.012	1
480			min	1.128	10	-345.213	3	-56.444	1	-.014	1	-.013	5	-.019	3
481		13	max	53.191	4	199.97	1	7.526	5	.015	3	-.001	10	.145	3
482			min	1.128	10	-248.007	3	-36.793	1	-.014	1	-.023	1	-.121	1
483		14	max	47.69	4	121.254	1	8.539	5	.015	3	-.003	15	.256	3
484			min	1.128	10	-150.801	3	-17.141	1	-.014	1	-.038	1	-.21	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	42.189	4	42.539	1	10.329	4	.015	3	0	5	.313	3
486			min	1.128	10	-53.595	3	-.842	10	-.014	1	-.042	1	-.255	1
487		16	max	36.689	4	43.611	3	22.161	1	.015	3	.006	5	.316	3
488			min	1.128	10	-36.176	1	.967	10	-.014	1	-.035	1	-.257	1
489		17	max	31.188	4	140.816	3	41.812	1	.015	3	.013	5	.264	3
490			min	1.128	10	-114.892	1	2.776	10	-.014	1	-.017	1	-.215	1
491		18	max	25.958	1	238.022	3	61.463	1	.015	3	.022	4	.159	3
492			min	1.128	10	-193.607	1	3.862	12	-.014	1	0	10	-.129	1
493		19	max	25.958	1	335.228	3	81.115	1	.015	3	.051	1	0	1
494			min	1.128	10	-272.322	1	4.5	12	-.014	1	.002	10	0	3
495	M16	1	max	38.235	5	341.248	2	7.339	5	.011	3	.05	1	0	2
496			min	-26.646	1	-155.071	3	-80.863	1	-.017	2	-.031	5	0	3
497		2	max	32.734	5	242.57	2	8.352	5	.011	3	.011	1	.074	3
498			min	-26.646	1	-110.61	3	-61.212	1	-.017	2	-.027	5	-.162	2
499		3	max	27.233	5	143.892	2	9.365	5	.011	3	0	12	.123	3
500			min	-26.646	1	-66.15	3	-41.561	1	-.017	2	-.025	4	-.27	2
501		4	max	21.732	5	45.215	2	10.378	5	.011	3	-.002	12	.147	3
502			min	-26.646	1	-21.69	3	-21.91	1	-.017	2	-.035	1	-.322	2
503		5	max	16.231	5	22.77	3	11.391	5	.011	3	-.002	12	.147	3
504			min	-26.646	1	-53.463	2	-2.258	1	-.017	2	-.042	1	-.32	2
505		6	max	10.73	5	67.23	3	17.393	1	.011	3	-.002	15	.122	3
506			min	-26.646	1	-152.14	2	-.681	3	-.017	2	-.038	1	-.263	2
507		7	max	5.229	5	111.69	3	37.044	1	.011	3	.004	5	.072	3
508			min	-26.646	1	-250.818	2	.247	12	-.017	2	-.023	1	-.151	2
509		8	max	1.38	3	156.15	3	56.695	1	.011	3	.012	4	.016	2
510			min	-26.646	1	-349.496	2	.886	12	-.017	2	-.004	3	-.002	3
511		9	max	1.38	3	200.61	3	76.346	1	.011	3	.04	1	.238	2
512			min	-26.646	1	-448.173	2	1.525	12	-.017	2	-.003	3	-.101	3
513		10	max	22.576	5	-9.958	15	95.998	1	.005	14	.088	1	.514	2
514			min	-26.646	1	-546.851	2	-3.782	3	-.017	2	.002	12	-.225	3
515		11	max	17.075	5	448.173	2	4.774	5	.017	2	.04	1	.238	2
516			min	-26.564	1	-200.61	3	-76.114	1	-.011	3	-.013	5	-.101	3
517		12	max	11.574	5	349.496	2	5.787	5	.017	2	.004	2	.016	2
518			min	-26.564	1	-156.15	3	-56.462	1	-.011	3	-.01	5	-.002	3
519		13	max	6.073	5	250.818	2	6.8	5	.017	2	0	12	.072	3
520			min	-26.564	1	-111.69	3	-36.811	1	-.011	3	-.023	1	-.151	2
521		14	max	.572	5	152.14	2	7.813	5	.017	2	-.001	12	.122	3
522			min	-26.564	1	-67.23	3	-17.16	1	-.011	3	-.038	1	-.263	2
523		15	max	-1.166	10	53.463	2	9.578	4	.017	2	.003	5	.147	3
524			min	-26.564	1	-22.77	3	-.843	10	-.011	3	-.042	1	-.32	2
525		16	max	-1.166	10	21.69	3	22.143	1	.017	2	.008	5	.147	3
526			min	-26.564	1	-45.215	2	.966	10	-.011	3	-.035	1	-.322	2
527		17	max	-1.166	10	66.15	3	41.794	1	.017	2	.014	5	.123	3
528			min	-26.564	1	-143.892	2	1.918	12	-.011	3	-.017	1	-.27	2
529		18	max	-1.166	10	110.61	3	61.445	1	.017	2	.023	4	.074	3
530			min	-27.075	4	-242.57	2	2.557	12	-.011	3	0	10	-.162	2
531		19	max	-1.166	10	155.071	3	81.096	1	.017	2	.051	1	0	2
532			min	-32.576	4	-341.248	2	3.196	12	-.011	3	.002	10	0	5
533	M15	1	max	0	1	1.025	3	.079	3	0	1	0	1	0	1
534			min	-61.419	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.911	3	.079	3	0	1	0	1	0	1
536			min	-61.478	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.798	3	.079	3	0	1	0	1	0	1
538			min	-61.538	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.684	3	.079	3	0	1	0	1	0	1
540			min	-61.598	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.57	3	.079	3	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-61.657	3	0	1	0	1	0	3	0	3	-.001	3
543		6	max	0	1	.456	3	.079	3	0	1	0	1	0	1
544			min	-61.717	3	0	1	0	1	0	3	0	3	-.001	3
545		7	max	0	1	.342	3	.079	3	0	1	0	3	0	1
546			min	-61.777	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.228	3	.079	3	0	1	0	3	0	1
548			min	-61.836	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.114	3	.079	3	0	1	0	3	0	1
550			min	-61.896	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.079	3	0	1	0	3	0	1
552			min	-61.956	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.079	3	0	1	0	3	0	1
554			min	-62.015	3	-.114	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.079	3	0	1	0	3	0	1
556			min	-62.075	3	-.228	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.079	3	0	1	0	3	0	1
558			min	-62.135	3	-.342	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.079	3	0	1	0	3	0	1
560			min	-62.194	3	-.456	3	0	1	0	3	0	1	-.001	3
561		15	max	0	1	0	1	.079	3	0	1	0	3	0	1
562			min	-62.254	3	-.57	3	0	1	0	3	0	1	-.001	3
563		16	max	0	1	0	1	.079	3	0	1	0	3	0	1
564			min	-62.314	3	-.684	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.079	3	0	1	0	3	0	1
566			min	-62.373	3	-.798	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.079	3	0	1	0	3	0	1
568			min	-62.433	3	-.911	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.079	3	0	1	0	3	0	1
570			min	-62.493	3	-1.025	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.367	4	.246	4	0	3	0	3	0	1
572			min	-188.543	4	0	2	-.036	3	0	1	0	4	0	1
573		2	max	0	2	2.104	4	.222	4	0	3	0	3	0	2
574			min	-188.571	4	0	2	-.036	3	0	1	0	4	0	4
575		3	max	0	2	1.841	4	.199	4	0	3	0	3	0	2
576			min	-188.599	4	0	2	-.036	3	0	1	0	4	-.001	4
577		4	max	0	2	1.578	4	.175	4	0	3	0	3	0	2
578			min	-188.626	4	0	2	-.036	3	0	1	0	1	-.002	4
579		5	max	0	2	1.315	4	.152	4	0	3	0	3	0	2
580			min	-188.654	4	0	2	-.036	3	0	1	0	1	-.002	4
581		6	max	0	2	1.052	4	.128	4	0	3	0	3	0	2
582			min	-188.682	4	0	2	-.036	3	0	1	0	1	-.003	4
583		7	max	0	2	.789	4	.105	4	0	3	0	3	0	2
584			min	-188.71	4	0	2	-.036	3	0	1	0	1	-.003	4
585		8	max	0	2	.526	4	.081	4	0	3	0	5	0	2
586			min	-188.738	4	0	2	-.036	3	0	1	0	1	-.003	4
587		9	max	0	2	.263	4	.058	4	0	3	0	5	0	2
588			min	-188.766	4	0	2	-.036	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.036	1	0	3	0	5	0	2
590			min	-188.794	4	0	1	-.036	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.036	1	0	3	0	5	0	2
592			min	-188.822	4	-.263	4	-.036	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.036	1	0	3	0	5	0	2
594			min	-188.85	4	-.526	4	-.036	3	0	1	0	1	-.003	4
595		13	max	0	2	0	2	.036	1	0	3	0	5	0	2
596			min	-188.878	4	-.789	4	-.04	5	0	1	0	9	-.003	4
597		14	max	0	2	0	2	.036	1	0	3	0	5	0	2
598			min	-188.905	4	-1.052	4	-.064	5	0	1	0	3	-.003	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	0	2	0	2	.036	1	0	3	0	4	0	2
600		min	-188.933	4	-1.315	4	-.087	5	0	1	0	3	-.002	4
601	16	max	0	2	0	2	.036	1	0	3	0	4	0	2
602		min	-188.961	4	-1.578	4	-.111	5	0	1	0	3	-.002	4
603	17	max	0	2	0	2	.036	1	0	3	0	1	0	2
604		min	-188.989	4	-1.841	4	-.134	5	0	1	0	3	-.001	4
605	18	max	.029	11	0	2	.036	1	0	3	0	1	0	2
606		min	-189.017	4	-2.104	4	-.158	5	0	1	0	3	0	4
607	19	max	.096	11	0	2	.036	1	0	3	0	1	0	1
608		min	-189.045	4	-2.367	4	-.181	5	0	1	0	5	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.007	2	.005	1	1.065e-3	5	NC	3	NC	2	
2			min	-.003	3	-.007	3	-.011	5	-3.803e-4	1	4502.178	2	6806.33	1	
3		2	max	.002	1	.007	2	.005	1	1.086e-3	5	NC	3	NC	2	
4			min	-.003	3	-.006	3	-.01	5	-3.646e-4	1	4884.721	2	7368.337	1	
5		3	max	.002	1	.006	2	.004	1	1.108e-3	5	NC	3	NC	2	
6			min	-.003	3	-.006	3	-.01	5	-3.49e-4	1	5334.752	2	8030.415	1	
7		4	max	.002	1	.006	2	.004	1	1.13e-3	5	NC	3	NC	2	
8			min	-.002	3	-.006	3	-.01	5	-3.334e-4	1	5867.761	2	8816.926	1	
9		5	max	.002	1	.005	2	.003	1	1.152e-3	5	NC	1	NC	2	
10			min	-.002	3	-.005	3	-.009	5	-3.178e-4	1	6504.166	2	9760.409	1	
11		6	max	.002	1	.005	2	.003	1	1.174e-3	5	NC	1	NC	1	
12			min	-.002	3	-.005	3	-.009	5	-3.021e-4	1	7271.321	2	NC	1	
13		7	max	.001	1	.004	2	.003	1	1.196e-3	5	NC	1	NC	1	
14			min	-.002	3	-.005	3	-.008	5	-2.865e-4	1	8206.564	2	NC	1	
15		8	max	.001	1	.004	2	.002	1	1.217e-3	5	NC	1	NC	1	
16			min	-.002	3	-.005	3	-.008	5	-2.709e-4	1	9361.979	2	NC	1	
17		9	max	.001	1	.003	2	.002	1	1.239e-3	5	NC	1	NC	1	
18			min	-.002	3	-.004	3	-.007	5	-2.553e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.002	1	1.261e-3	5	NC	1	NC	1	
20			min	-.001	3	-.004	3	-.007	5	-2.396e-4	1	NC	1	NC	1	
21		11	max	0	1	.002	2	.001	1	1.283e-3	5	NC	1	NC	1	
22			min	-.001	3	-.004	3	-.006	5	-2.24e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.001	1	1.305e-3	5	NC	1	NC	1	
24			min	-.001	3	-.003	3	-.005	5	-2.084e-4	1	NC	1	NC	1	
25		13	max	0	1	.001	2	0	1	1.327e-3	5	NC	1	NC	1	
26			min	0	3	-.003	3	-.005	5	-1.928e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	0	1	1.349e-3	5	NC	1	NC	1	
28			min	0	3	-.002	3	-.004	5	-1.771e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	1.37e-3	5	NC	1	NC	1	
30			min	0	3	-.002	3	-.003	5	-1.615e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	1.392e-3	5	NC	1	NC	1	
32			min	0	3	-.002	3	-.002	5	-1.459e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	1.414e-3	5	NC	1	NC	1	
34			min	0	3	-.001	3	-.002	5	-1.303e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	1.436e-3	5	NC	1	NC	1	
36			min	0	3	0	3	0	5	-1.146e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	1.458e-3	5	NC	1	NC	1	
38			min	0	1	0	1	0	1	-9.9e-5	1	NC	1	NC	1	
39		M3	1	max	0	1	0	1	0	1	4.55e-5	1	NC	1	NC	1
40				min	0	1	0	1	0	1	-6.701e-4	5	NC	1	NC	1
41	2		max	0	3	0	2	.004	5	5.807e-5	1	NC	1	NC	1	
42			min	0	2	0	3	0	1	-6.734e-4	5	NC	1	NC	1	



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.007	5	7.063e-5	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	-6.767e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.011	5	8.32e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.8e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.014	5	9.577e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.833e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.018	4	1.083e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	0	1	-6.866e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.021	4	1.209e-4	1	NC	1	NC	1
52			min	0	2	-.004	3	0	1	-6.899e-4	5	NC	1	NC	1
53		8	max	0	3	.001	2	.025	4	1.335e-4	1	NC	1	NC	1
54			min	0	2	-.005	3	0	9	-6.932e-4	5	NC	1	NC	1
55		9	max	0	3	.001	2	.028	4	1.46e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	9	-6.965e-4	5	NC	1	NC	1
57		10	max	0	3	.002	2	.031	4	1.586e-4	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-6.998e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.035	4	1.712e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-7.03e-4	5	NC	1	NC	1
61		12	max	0	3	.003	2	.038	4	1.838e-4	1	NC	1	NC	1
62			min	0	2	-.007	3	0	10	-7.063e-4	5	NC	1	NC	1
63		13	max	0	3	.004	2	.041	4	1.963e-4	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-7.096e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.044	4	2.089e-4	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-7.129e-4	5	NC	1	NC	1
67		15	max	0	3	.005	2	.047	4	2.215e-4	1	NC	1	NC	1
68			min	0	2	-.008	3	0	10	-7.162e-4	5	8702.481	2	NC	1
69		16	max	0	3	.006	2	.05	4	2.34e-4	1	NC	1	NC	1
70			min	0	2	-.008	3	0	10	-7.195e-4	5	7416.163	2	NC	1
71		17	max	0	3	.007	2	.052	4	2.466e-4	1	NC	3	NC	1
72			min	0	2	-.008	3	0	10	-7.228e-4	5	6410.811	2	NC	1
73		18	max	0	3	.008	2	.055	4	2.592e-4	1	NC	3	NC	1
74			min	0	2	-.008	3	0	10	-7.261e-4	5	5617.799	2	NC	1
75		19	max	0	3	.009	2	.058	4	2.717e-4	1	NC	3	NC	1
76			min	0	2	-.008	3	0	10	-7.294e-4	5	4987.748	2	NC	1
77	M4	1	max	.002	1	.008	2	0	10	2.919e-3	5	NC	1	NC	2
78			min	0	3	-.007	3	-.061	4	-3.267e-4	1	NC	1	316.426	4
79		2	max	.002	1	.008	2	0	10	2.919e-3	5	NC	1	NC	1
80			min	0	3	-.006	3	-.056	4	-3.267e-4	1	NC	1	344.923	4
81		3	max	.002	1	.007	2	0	10	2.919e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.051	4	-3.267e-4	1	NC	1	378.838	4
83		4	max	.001	1	.007	2	0	10	2.919e-3	5	NC	1	NC	1
84			min	0	3	-.006	3	-.046	4	-3.267e-4	1	NC	1	419.598	4
85		5	max	.001	1	.007	2	0	10	2.919e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.041	4	-3.267e-4	1	NC	1	469.146	4
87		6	max	.001	1	.006	2	0	10	2.919e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.036	4	-3.267e-4	1	NC	1	530.185	4
89		7	max	.001	1	.006	2	0	10	2.919e-3	5	NC	1	NC	1
90			min	0	3	-.005	3	-.032	4	-3.267e-4	1	NC	1	606.565	4
91		8	max	.001	1	.005	2	0	10	2.919e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.027	4	-3.267e-4	1	NC	1	703.923	4
93		9	max	0	1	.005	2	0	10	2.919e-3	5	NC	1	NC	1
94			min	0	3	-.004	3	-.023	4	-3.267e-4	1	NC	1	830.782	4
95		10	max	0	1	.004	2	0	10	2.919e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.019	4	-3.267e-4	1	NC	1	1000.545	4
97		11	max	0	1	.004	2	0	10	2.919e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.016	4	-3.267e-4	1	NC	1	1235.28	4
99		12	max	0	1	.003	2	0	10	2.919e-3	5	NC	1	NC	1



Company : Schletter, Inc.
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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.003	3	-.012	4	-3.267e-4	1	NC	1	1573.483	4
101		max	0	1	.003	2	0	10	2.919e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.009	4	-3.267e-4	1	NC	1	2087.356	4
103		max	0	1	.002	2	0	10	2.919e-3	5	NC	1	NC	1
104		min	0	3	-.002	3	-.007	4	-3.267e-4	1	NC	1	2925.83	4
105		max	0	1	.002	2	0	10	2.919e-3	5	NC	1	NC	1
106		min	0	3	-.002	3	-.004	4	-3.267e-4	1	NC	1	4438.778	4
107		max	0	1	.001	2	0	10	2.919e-3	5	NC	1	NC	1
108		min	0	3	-.001	3	-.003	4	-3.267e-4	1	NC	1	7620.778	4
109		max	0	1	0	2	0	10	2.919e-3	5	NC	1	NC	1
110		min	0	3	0	3	-.001	4	-3.267e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.919e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-3.267e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.919e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-3.267e-4	1	NC	1	NC	1
115	M6	max	.007	1	.021	2	.002	1	1.145e-3	4	NC	3	NC	1
116		min	-.009	3	-.017	3	-.011	5	-8.042e-8	2	1601.821	2	8801.94	3
117		max	.007	1	.019	2	.002	1	1.166e-3	4	NC	3	NC	1
118		min	-.009	3	-.016	3	-.01	5	-7.602e-8	2	1712.743	2	9416.113	3
119		max	.006	1	.018	2	.002	1	1.187e-3	4	NC	3	NC	1
120		min	-.008	3	-.015	3	-.01	5	-7.162e-8	2	1839.705	2	NC	1
121		max	.006	1	.017	2	.001	1	1.208e-3	4	NC	3	NC	1
122		min	-.008	3	-.014	3	-.01	5	-3.862e-7	11	1985.937	2	NC	1
123		max	.005	1	.015	2	.001	1	1.229e-3	4	NC	3	NC	1
124		min	-.007	3	-.014	3	-.009	5	-2.231e-6	1	2155.607	2	NC	1
125		max	.005	1	.014	2	.001	1	1.25e-3	4	NC	3	NC	1
126		min	-.007	3	-.013	3	-.009	5	-5.561e-6	1	2354.171	2	NC	1
127		max	.005	1	.013	2	.001	1	1.271e-3	4	NC	3	NC	1
128		min	-.006	3	-.012	3	-.008	5	-8.891e-6	1	2588.916	2	NC	1
129		max	.004	1	.012	2	0	1	1.292e-3	4	NC	3	NC	1
130		min	-.006	3	-.011	3	-.008	5	-1.222e-5	1	2869.789	2	NC	1
131		max	.004	1	.01	2	0	1	1.313e-3	4	NC	3	NC	1
132		min	-.005	3	-.01	3	-.007	5	-1.555e-5	1	3210.728	2	NC	1
133		max	.004	1	.009	2	0	1	1.334e-3	4	NC	3	NC	1
134		min	-.005	3	-.009	3	-.007	5	-1.888e-5	1	3631.874	2	NC	1
135		max	.003	1	.008	2	0	1	1.355e-3	4	NC	3	NC	1
136		min	-.004	3	-.008	3	-.006	5	-2.221e-5	1	4163.453	2	NC	1
137		max	.003	1	.007	2	0	1	1.376e-3	4	NC	3	NC	1
138		min	-.004	3	-.007	3	-.005	5	-2.554e-5	1	4852.97	2	NC	1
139		max	.002	1	.006	2	0	1	1.397e-3	4	NC	3	NC	1
140		min	-.003	3	-.006	3	-.005	5	-2.887e-5	1	5779.616	2	NC	1
141		max	.002	1	.005	2	0	1	1.418e-3	4	NC	3	NC	1
142		min	-.003	3	-.005	3	-.004	5	-3.22e-5	1	7085.941	2	NC	1
143		max	.002	1	.004	2	0	1	1.439e-3	4	NC	1	NC	1
144		min	-.002	3	-.004	3	-.003	5	-3.553e-5	1	9057.052	2	NC	1
145		max	.001	1	.003	2	0	1	1.46e-3	4	NC	1	NC	1
146		min	-.002	3	-.003	3	-.002	5	-3.886e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.481e-3	4	NC	1	NC	1
148		min	-.001	3	-.002	3	-.002	5	-4.219e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.502e-3	4	NC	1	NC	1
150		min	0	3	-.001	3	0	5	-4.552e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.523e-3	5	NC	1	NC	1
152		min	0	1	0	1	0	1	-4.885e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	2.229e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.998e-4	4	NC	1	NC	1
155		max	0	3	.001	2	.004	4	1.963e-5	1	NC	1	NC	1
156		min	0	2	-.002	3	0	1	-6.894e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.007	4	1.697e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-6.79e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.011	4	1.431e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-6.686e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.015	4	1.165e-5	1	NC	1	NC	1
162			min	0	2	-.006	3	0	1	-6.582e-4	4	NC	1	NC	1
163		6	max	0	3	.005	2	.019	4	1.035e-5	3	NC	1	NC	1
164			min	-.001	2	-.007	3	0	1	-6.478e-4	4	8658.808	2	NC	1
165		7	max	0	3	.006	2	.022	4	2.628e-5	3	NC	3	NC	1
166			min	-.001	2	-.009	3	0	1	-6.374e-4	4	7179.139	2	NC	1
167		8	max	.001	3	.008	2	.026	4	4.222e-5	3	NC	3	NC	1
168			min	-.001	2	-.01	3	0	1	-6.27e-4	4	6088.474	2	NC	1
169		9	max	.001	3	.009	2	.029	4	5.816e-5	3	NC	3	NC	1
170			min	-.002	2	-.011	3	0	1	-6.166e-4	4	5246.375	2	NC	1
171		10	max	.001	3	.01	2	.033	4	7.409e-5	3	NC	3	NC	1
172			min	-.002	2	-.012	3	0	1	-6.062e-4	4	4575.022	2	NC	1
173		11	max	.002	3	.011	2	.036	4	9.003e-5	3	NC	3	NC	1
174			min	-.002	2	-.014	3	0	1	-5.958e-4	4	4027.591	2	NC	1
175		12	max	.002	3	.013	2	.039	4	1.06e-4	3	NC	3	NC	1
176			min	-.002	2	-.015	3	0	1	-5.854e-4	4	3573.959	2	NC	1
177		13	max	.002	3	.014	2	.042	4	1.219e-4	3	NC	3	NC	1
178			min	-.002	2	-.015	3	0	1	-5.75e-4	4	3193.635	2	NC	1
179		14	max	.002	3	.016	2	.045	4	1.378e-4	3	NC	3	NC	1
180			min	-.003	2	-.016	3	0	1	-5.646e-4	4	2872.021	2	NC	1
181		15	max	.002	3	.018	2	.048	4	1.538e-4	3	NC	3	NC	1
182			min	-.003	2	-.017	3	0	1	-5.542e-4	4	2598.313	2	NC	1
183		16	max	.002	3	.019	2	.051	4	1.697e-4	3	NC	3	NC	1
184			min	-.003	2	-.018	3	-.001	1	-5.438e-4	4	2364.269	2	NC	1
185		17	max	.002	3	.021	2	.054	4	1.857e-4	3	NC	3	NC	1
186			min	-.003	2	-.019	3	-.001	1	-5.334e-4	4	2163.454	2	NC	1
187		18	max	.003	3	.023	2	.057	4	2.016e-4	3	NC	3	NC	1
188			min	-.003	2	-.019	3	-.001	1	-5.23e-4	4	1990.757	2	NC	1
189		19	max	.003	3	.025	2	.059	4	2.175e-4	3	NC	3	NC	1
190			min	-.004	2	-.02	3	-.001	1	-5.126e-4	4	1842.069	2	NC	1
191	M8	1	max	.005	1	.024	2	.001	1	2.723e-3	4	NC	1	NC	1
192			min	-.001	3	-.018	3	-.062	4	-1.668e-4	3	NC	1	310.83	4
193		2	max	.005	1	.022	2	0	1	2.723e-3	4	NC	1	NC	1
194			min	-.001	3	-.017	3	-.057	4	-1.668e-4	3	NC	1	338.823	4
195		3	max	.005	1	.021	2	0	1	2.723e-3	4	NC	1	NC	1
196			min	-.001	3	-.016	3	-.052	4	-1.668e-4	3	NC	1	372.14	4
197		4	max	.004	1	.02	2	0	1	2.723e-3	4	NC	1	NC	1
198			min	-.001	3	-.015	3	-.047	4	-1.668e-4	3	NC	1	412.18	4
199		5	max	.004	1	.018	2	0	1	2.723e-3	4	NC	1	NC	1
200			min	0	3	-.014	3	-.042	4	-1.668e-4	3	NC	1	460.854	4
201		6	max	.004	1	.017	2	0	1	2.723e-3	4	NC	1	NC	1
202			min	0	3	-.013	3	-.037	4	-1.668e-4	3	NC	1	520.817	4
203		7	max	.003	1	.016	2	0	1	2.723e-3	4	NC	1	NC	1
204			min	0	3	-.012	3	-.032	4	-1.668e-4	3	NC	1	595.85	4
205		8	max	.003	1	.014	2	0	1	2.723e-3	4	NC	1	NC	1
206			min	0	3	-.011	3	-.028	4	-1.668e-4	3	NC	1	691.491	4
207		9	max	.003	1	.013	2	0	1	2.723e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.024	4	-1.668e-4	3	NC	1	816.115	4
209		10	max	.003	1	.012	2	0	1	2.723e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.02	4	-1.668e-4	3	NC	1	982.886	4
211		11	max	.002	1	.011	2	0	1	2.723e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.016	4	-1.668e-4	3	NC	1	1213.486	4
213		12	max	.002	1	.009	2	0	1	2.723e-3	4	NC	1	NC	1





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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.006	4	4.041e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-6.337e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.009	4	2.424e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-6.869e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.011	4	8.068e-6	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-7.401e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.014	4	-4.755e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.001	3	-7.933e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.017	5	-5.416e-6	10	NC	1	NC	1
280			min	0	2	-.005	3	-.002	3	-8.465e-4	4	NC	1	NC	1
281		8	max	0	3	.001	2	.02	5	-6.078e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-8.998e-4	4	NC	1	NC	1
283		9	max	0	3	.001	2	.023	5	-6.739e-6	10	NC	1	NC	1
284			min	0	2	-.006	3	-.002	3	-9.53e-4	4	NC	1	NC	1
285		10	max	0	3	.002	2	.026	5	-7.401e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-1.006e-3	4	NC	1	NC	1
287		11	max	0	3	.002	2	.029	5	-8.062e-6	10	NC	1	NC	1
288			min	0	2	-.007	3	-.002	3	-1.059e-3	4	NC	1	NC	1
289		12	max	0	3	.003	2	.032	5	-8.723e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.002	1	-1.113e-3	4	NC	1	NC	1
291		13	max	0	3	.004	2	.035	5	-9.385e-6	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	1	-1.166e-3	4	NC	1	NC	1
293		14	max	0	3	.004	2	.038	5	-1.005e-5	10	NC	1	NC	1
294			min	0	2	-.008	3	-.003	1	-1.219e-3	4	NC	1	NC	1
295		15	max	0	3	.005	2	.041	5	-1.071e-5	10	NC	1	NC	1
296			min	0	2	-.008	3	-.003	1	-1.272e-3	4	8714.444	2	NC	1
297		16	max	0	3	.006	2	.043	5	-1.137e-5	10	NC	1	NC	1
298			min	0	2	-.008	3	-.004	1	-1.325e-3	4	7425.325	2	NC	1
299		17	max	0	3	.007	2	.046	5	-1.203e-5	10	NC	3	NC	1
300			min	0	2	-.008	3	-.004	1	-1.379e-3	4	6418.005	2	NC	1
301		18	max	0	3	.008	2	.049	5	-1.269e-5	10	NC	3	NC	1
302			min	0	2	-.008	3	-.005	1	-1.432e-3	4	5623.588	2	NC	1
303		19	max	0	3	.009	2	.052	5	-1.335e-5	10	NC	3	NC	2
304			min	0	2	-.008	3	-.005	1	-1.485e-3	4	4992.519	2	9550.286	1
305	M12	1	max	.002	1	.008	2	.004	1	3.551e-3	4	NC	1	NC	2
306			min	0	3	-.007	3	-.057	5	1.344e-5	10	NC	1	338.31	5
307		2	max	.002	1	.008	2	.004	1	3.551e-3	4	NC	1	NC	2
308			min	0	3	-.006	3	-.052	5	1.344e-5	10	NC	1	368.771	5
309		3	max	.002	1	.007	2	.003	1	3.551e-3	4	NC	1	NC	2
310			min	0	3	-.006	3	-.048	5	1.344e-5	10	NC	1	405.023	5
311		4	max	.001	1	.007	2	.003	1	3.551e-3	4	NC	1	NC	2
312			min	0	3	-.006	3	-.043	5	1.344e-5	10	NC	1	448.59	5
313		5	max	.001	1	.007	2	.003	1	3.551e-3	4	NC	1	NC	2
314			min	0	3	-.005	3	-.039	5	1.344e-5	10	NC	1	501.549	5
315		6	max	.001	1	.006	2	.002	1	3.551e-3	4	NC	1	NC	2
316			min	0	3	-.005	3	-.034	5	1.344e-5	10	NC	1	566.79	5
317		7	max	.001	1	.006	2	.002	1	3.551e-3	4	NC	1	NC	2
318			min	0	3	-.005	3	-.03	5	1.344e-5	10	NC	1	648.427	5
319		8	max	.001	1	.005	2	.002	1	3.551e-3	4	NC	1	NC	1
320			min	0	3	-.004	3	-.026	5	1.344e-5	10	NC	1	752.483	5
321		9	max	0	1	.005	2	.002	1	3.551e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.022	5	1.344e-5	10	NC	1	888.07	5
323		10	max	0	1	.004	2	.001	1	3.551e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.018	5	1.344e-5	10	NC	1	1069.507	5
325		11	max	0	1	.004	2	.001	1	3.551e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.015	5	1.344e-5	10	NC	1	1320.382	5
327		12	max	0	1	.003	2	0	1	3.551e-3	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	3	-.003	3	-.011	5	1.344e-5	10	NC	1	1681.832	5
329		max	0	1	.003	2	0	1	3.551e-3	4	NC	1	NC	1
330		min	0	3	-.002	3	-.009	5	1.344e-5	10	NC	1	2231.019	5
331		max	0	1	.002	2	0	1	3.551e-3	4	NC	1	NC	1
332		min	0	3	-.002	3	-.006	5	1.344e-5	10	NC	1	3127.1	5
333		max	0	1	.002	2	0	1	3.551e-3	4	NC	1	NC	1
334		min	0	3	-.002	3	-.004	5	1.344e-5	10	NC	1	4743.966	5
335		max	0	1	.001	2	0	1	3.551e-3	4	NC	1	NC	1
336		min	0	3	-.001	3	-.002	5	1.344e-5	10	NC	1	8144.467	5
337		max	0	1	0	2	0	1	3.551e-3	4	NC	1	NC	1
338		min	0	3	0	3	-.001	5	1.344e-5	10	NC	1	NC	1
339		max	0	1	0	2	0	1	3.551e-3	4	NC	1	NC	1
340		min	0	3	0	3	0	5	1.344e-5	10	NC	1	NC	1
341		max	0	1	0	1	0	1	3.551e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.344e-5	10	NC	1	NC	1
343	M1	max	.006	3	.023	3	.006	5	5.372e-3	1	NC	1	NC	1
344		min	-.007	2	-.022	1	-.002	1	-6.474e-3	3	NC	1	NC	1
345		max	.006	3	.013	3	.008	5	2.549e-3	1	NC	4	NC	1
346		min	-.007	2	-.012	1	-.004	1	-3.182e-3	3	4558.455	2	NC	1
347		max	.006	3	.003	3	.011	5	2.791e-4	5	NC	4	NC	1
348		min	-.007	2	-.002	1	-.005	1	-2.215e-4	1	2349.809	2	9730.898	5
349		max	.006	3	.006	2	.014	5	2.767e-4	5	NC	4	NC	1
350		min	-.007	2	-.005	3	-.006	1	-1.846e-4	1	1649.12	2	6077.991	5
351		max	.006	3	.013	2	.017	5	2.743e-4	5	NC	5	NC	1
352		min	-.007	2	-.012	3	-.006	1	-1.478e-4	1	1310.368	2	4313.154	5
353		max	.006	3	.019	2	.02	5	2.719e-4	5	NC	5	NC	1
354		min	-.007	2	-.017	3	-.005	1	-1.11e-4	1	1117.254	2	3291.498	5
355		max	.006	3	.024	2	.024	5	2.696e-4	5	NC	5	NC	1
356		min	-.007	2	-.021	3	-.005	1	-7.412e-5	1	998.637	2	2634.97	5
357		max	.006	3	.027	2	.028	5	2.672e-4	5	NC	5	NC	1
358		min	-.007	2	-.024	3	-.004	1	-3.729e-5	1	924.814	2	2183.13	5
359		max	.006	3	.029	2	.031	5	2.648e-4	5	NC	5	NC	1
360		min	-.007	2	-.025	3	-.003	1	-7.781e-6	9	881.791	2	1854.464	4
361		max	.006	3	.03	2	.035	5	2.704e-4	4	NC	5	NC	1
362		min	-.007	2	-.026	3	-.001	1	4.171e-6	10	862.939	2	1595.083	4
363		max	.006	3	.029	2	.039	4	2.761e-4	4	NC	5	NC	1
364		min	-.007	2	-.025	3	0	9	5.61e-6	10	865.948	2	1398.758	4
365		max	.006	3	.028	2	.043	4	2.818e-4	4	NC	5	NC	1
366		min	-.007	2	-.023	3	0	10	7.049e-6	10	891.945	2	1246.866	4
367		max	.006	3	.024	2	.047	4	2.875e-4	4	NC	5	NC	1
368		min	-.007	2	-.02	3	0	10	8.488e-6	10	946.059	2	1127.387	4
369		max	.006	3	.019	2	.051	4	2.932e-4	4	NC	5	NC	1
370		min	-.007	2	-.015	3	0	10	9.927e-6	10	1039.973	2	1032.275	4
371		max	.006	3	.013	2	.055	4	2.989e-4	4	NC	4	NC	1
372		min	-.007	2	-.01	3	0	10	1.137e-5	10	1199.151	2	955.991	4
373		max	.006	3	.005	2	.058	4	4.978e-4	4	NC	4	NC	1
374		min	-.007	2	-.004	3	0	10	1.246e-5	10	1485.323	2	894.629	4
375		max	.006	3	.003	3	.061	4	5.295e-3	4	NC	4	NC	1
376		min	-.007	2	-.004	2	0	10	5.414e-6	10	2091.58	2	845.451	4
377		max	.006	3	.01	3	.064	4	3.344e-3	2	NC	4	NC	1
378		min	-.007	2	-.015	2	0	10	-1.576e-3	3	4032.061	2	806.324	4
379		max	.006	3	.018	3	.066	4	6.714e-3	2	NC	1	NC	1
380		min	-.007	2	-.027	2	-.001	1	-3.221e-3	3	NC	1	776.66	4
381	M5	max	.016	3	.064	3	.006	5	9.101e-6	4	NC	1	NC	1
382		min	-.02	2	-.06	1	-.002	1	4.217e-8	11	NC	1	NC	1
383		max	.016	3	.036	3	.008	5	1.355e-4	5	NC	4	NC	1
384		min	-.02	2	-.033	1	-.002	1	-3.712e-5	1	1704.558	1	NC	1



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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.016	3	.01	3	.011	5	2.599e-4	5	NC	5	NC	1
386		min	-.02	2	-.007	1	-.002	1	-7.356e-5	1	876.129	1	NC	1
387	4	max	.016	3	.015	2	.014	5	2.711e-4	5	NC	5	NC	1
388		min	-.02	2	-.012	3	-.002	1	-6.946e-5	1	617.255	1	NC	1
389	5	max	.016	3	.034	2	.017	5	2.823e-4	5	NC	5	NC	1
390		min	-.02	2	-.03	3	-.002	1	-6.537e-5	1	489.999	2	NC	1
391	6	max	.016	3	.05	2	.021	5	2.935e-4	5	NC	5	NC	1
392		min	-.02	2	-.044	3	-.002	1	-6.128e-5	1	417.051	2	NC	1
393	7	max	.016	3	.063	2	.025	5	3.047e-4	5	NC	5	NC	1
394		min	-.02	2	-.055	3	-.002	1	-5.718e-5	1	372.177	2	NC	1
395	8	max	.016	3	.072	2	.029	5	3.159e-4	5	NC	5	NC	1
396		min	-.02	2	-.062	3	-.002	1	-5.309e-5	1	344.158	2	NC	1
397	9	max	.016	3	.078	2	.033	5	3.272e-4	5	NC	5	NC	1
398		min	-.02	2	-.066	3	-.001	1	-4.9e-5	1	327.704	2	NC	1
399	10	max	.016	3	.08	2	.037	5	3.384e-4	5	NC	5	NC	1
400		min	-.02	2	-.067	3	-.001	1	-4.49e-5	1	320.303	2	NC	1
401	11	max	.016	3	.079	2	.041	5	3.496e-4	5	NC	5	NC	1
402		min	-.02	2	-.064	3	-.001	1	-4.081e-5	1	321.062	2	NC	1
403	12	max	.016	3	.074	2	.045	4	3.608e-4	5	NC	5	NC	1
404		min	-.02	2	-.059	3	-.001	1	-3.671e-5	1	330.378	2	NC	1
405	13	max	.016	3	.065	2	.049	4	3.72e-4	5	NC	5	NC	1
406		min	-.02	2	-.051	3	-.001	1	-3.262e-5	1	350.137	2	NC	1
407	14	max	.016	3	.052	2	.053	4	3.832e-4	5	NC	5	NC	1
408		min	-.021	2	-.04	3	-.001	1	-2.853e-5	1	384.664	2	NC	1
409	15	max	.016	3	.035	2	.056	4	3.944e-4	5	NC	5	NC	1
410		min	-.021	2	-.027	3	-.001	1	-2.443e-5	1	443.42	2	NC	1
411	16	max	.016	3	.014	2	.059	4	5.952e-4	4	NC	5	NC	1
412		min	-.021	2	-.011	3	-.001	1	-2.274e-5	1	549.43	2	NC	1
413	17	max	.016	3	.007	3	.062	4	5.325e-3	4	NC	5	NC	1
414		min	-.021	2	-.012	2	-.001	1	-7.822e-5	1	775.405	2	NC	1
415	18	max	.016	3	.027	3	.064	4	2.733e-3	4	NC	4	NC	1
416		min	-.021	2	-.042	2	0	1	-4.002e-5	1	1503.99	2	NC	1
417	19	max	.016	3	.047	3	.066	4	3.515e-6	5	NC	1	NC	1
418		min	-.021	2	-.074	2	0	1	-3.665e-7	3	NC	1	NC	1
419	M9	1	max	.006	.023	3	.005	5	6.479e-3	3	NC	1	NC	1
420		min	-.007	2	-.022	1	-.002	1	-5.372e-3	1	NC	1	NC	1
421	2	max	.006	3	.013	3	.005	5	3.211e-3	3	NC	4	NC	1
422		min	-.007	2	-.012	1	0	9	-2.629e-3	1	4559.216	2	NC	1
423	3	max	.006	3	.003	3	.005	4	6.214e-5	1	NC	4	NC	1
424		min	-.007	2	-.002	1	0	3	-2.153e-5	5	2350.214	2	NC	1
425	4	max	.006	3	.006	2	.006	4	3.569e-5	2	NC	4	NC	1
426		min	-.007	2	-.005	3	-.001	3	-2.922e-5	5	1649.42	2	NC	1
427	5	max	.006	3	.013	2	.007	4	2.493e-5	2	NC	4	NC	1
428		min	-.007	2	-.012	3	-.002	3	-3.823e-5	4	1310.62	2	NC	1
429	6	max	.006	3	.019	2	.009	4	1.417e-5	2	NC	5	NC	1
430		min	-.007	2	-.017	3	-.002	3	-5.19e-5	4	1117.479	2	9814.471	4
431	7	max	.006	3	.024	2	.012	4	3.413e-6	2	NC	5	NC	1
432		min	-.007	2	-.021	3	-.003	3	-6.558e-5	4	998.848	2	6188.262	4
433	8	max	.006	3	.027	2	.015	4	-1.165e-6	10	NC	5	NC	1
434		min	-.007	2	-.024	3	-.003	3	-8.763e-5	1	925.018	2	4315.651	4
435	9	max	.006	3	.029	2	.019	4	-2.612e-6	10	NC	5	NC	1
436		min	-.007	2	-.025	3	-.003	3	-1.176e-4	1	881.993	2	3216.829	4
437	10	max	.006	3	.03	2	.023	5	-4.058e-6	10	NC	5	NC	1
438		min	-.007	2	-.026	3	-.003	3	-1.475e-4	1	863.144	2	2514.559	4
439	11	max	.006	3	.029	2	.028	5	-5.505e-6	10	NC	5	NC	1
440		min	-.007	2	-.025	3	-.003	3	-1.775e-4	1	866.161	2	2037.385	4
441	12	max	.006	3	.028	2	.033	5	-6.951e-6	10	NC	5	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	0	1	.057	3	.015	4	6.228e-3	2	NC	4	NC	2
500		min	-.066	4	-.107	2	-.004	10	-4.145e-3	3	1506.861	2	6237.135	1
501	4	max	0	1	.069	3	.022	1	7.062e-3	2	NC	5	NC	2
502		min	-.066	4	-.131	2	-.004	10	-4.667e-3	3	1150.223	2	4407.384	1
503	5	max	0	1	.075	3	.025	1	7.897e-3	2	NC	5	NC	2
504		min	-.066	4	-.142	2	-.005	10	-5.19e-3	3	1043.033	2	3985.277	1
505	6	max	0	1	.075	3	.022	1	8.732e-3	2	NC	5	NC	2
506		min	-.066	4	-.139	2	-.006	10	-5.712e-3	3	1072.375	2	4461.298	1
507	7	max	0	1	.069	3	.017	3	9.567e-3	2	NC	5	NC	2
508		min	-.066	4	-.125	2	-.008	10	-6.234e-3	3	1232.07	2	6589.657	1
509	8	max	0	1	.06	3	.018	3	1.04e-2	2	NC	4	NC	1
510		min	-.066	4	-.104	2	-.013	2	-6.757e-3	3	1569.402	2	NC	1
511	9	max	0	1	.051	3	.017	3	1.124e-2	2	NC	4	NC	1
512		min	-.066	4	-.084	2	-.018	2	-7.279e-3	3	2126.356	2	NC	1
513	10	max	0	1	.047	3	.016	3	1.207e-2	2	NC	4	NC	1
514		min	-.066	4	-.074	2	-.021	2	-7.801e-3	3	2549.699	2	9072.242	2
515	11	max	0	1	.051	3	.015	3	1.124e-2	2	NC	4	NC	1
516		min	-.066	4	-.084	2	-.018	2	-7.278e-3	3	2126.356	2	NC	1
517	12	max	.001	1	.06	3	.015	3	1.04e-2	2	NC	4	NC	1
518		min	-.066	4	-.104	2	-.013	2	-6.755e-3	3	1569.402	2	NC	1
519	13	max	.001	1	.069	3	.014	3	9.568e-3	2	NC	5	NC	2
520		min	-.066	4	-.125	2	-.008	10	-6.232e-3	3	1232.07	2	6584.778	1
521	14	max	.001	1	.075	3	.022	1	8.733e-3	2	NC	5	NC	2
522		min	-.066	4	-.139	2	-.006	10	-5.709e-3	3	1072.375	2	4469.408	1
523	15	max	.001	1	.075	3	.025	1	7.898e-3	2	NC	5	NC	2
524		min	-.066	4	-.142	2	-.005	10	-5.186e-3	3	1043.033	2	4000.236	1
525	16	max	.001	1	.069	3	.022	1	7.063e-3	2	NC	5	NC	2
526		min	-.066	4	-.131	2	-.005	5	-4.663e-3	3	1150.223	2	4433.235	1
527	17	max	.001	1	.057	3	.014	1	6.229e-3	2	NC	4	NC	2
528		min	-.066	4	-.107	2	-.005	5	-4.139e-3	3	1506.861	2	6292.125	1
529	18	max	.001	1	.039	3	.007	3	5.394e-3	2	NC	4	NC	1
530		min	-.066	4	-.07	2	-.005	2	-3.616e-3	3	2772.305	2	NC	1
531	19	max	.001	1	.018	3	.006	3	4.559e-3	2	NC	1	NC	1
532		min	-.066	4	-.027	2	-.007	2	-3.093e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.427e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-4.908e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.005	4	7.621e-4	3	NC	1	NC	1
536		min	0	4	-.004	1	0	3	-5.081e-4	5	NC	1	NC	1
537	3	max	0	3	0	5	.01	4	1.181e-3	3	NC	3	NC	1
538		min	-.001	4	-.007	1	-.003	3	-9.357e-4	2	9766.427	1	6782.421	4
539	4	max	0	3	0	5	.015	4	1.601e-3	3	NC	5	NC	9
540		min	-.002	4	-.011	1	-.006	3	-1.366e-3	2	6700.34	1	4442.951	4
541	5	max	0	3	0	5	.02	4	2.02e-3	3	NC	5	NC	9
542		min	-.002	4	-.013	1	-.009	3	-1.796e-3	2	5228.343	1	3371.958	4
543	6	max	0	3	.001	5	.024	4	2.439e-3	3	NC	5	9154.531	9
544		min	-.003	4	-.016	1	-.014	3	-2.227e-3	2	4400.203	1	2802.402	4
545	7	max	0	3	.001	5	.028	4	2.859e-3	3	NC	5	7163.527	9
546		min	-.003	4	-.018	1	-.018	3	-2.666e-3	1	3902.187	1	2487.386	4
547	8	max	0	3	.002	5	.029	4	3.278e-3	3	NC	5	5910.922	9
548		min	-.004	4	-.02	1	-.022	3	-3.106e-3	1	3603.302	1	2326.519	3
549	9	max	0	3	.002	5	.03	4	3.697e-3	3	NC	5	5090.803	9
550		min	-.004	4	-.021	1	-.026	3	-3.546e-3	1	3442.424	1	1999.051	3
551	10	max	0	3	.002	5	.029	4	4.117e-3	3	NC	5	4549.397	9
552		min	-.005	4	-.021	1	-.029	3	-3.985e-3	1	3391.53	1	1783.087	3
553	11	max	0	3	.002	5	.03	1	4.536e-3	3	NC	5	4656.102	15
554		min	-.005	4	-.021	1	-.031	3	-4.425e-3	1	3442.424	1	1645.914	3
555	12	max	0	3	.003	5	.031	1	4.956e-3	3	NC	5	5495.363	15





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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

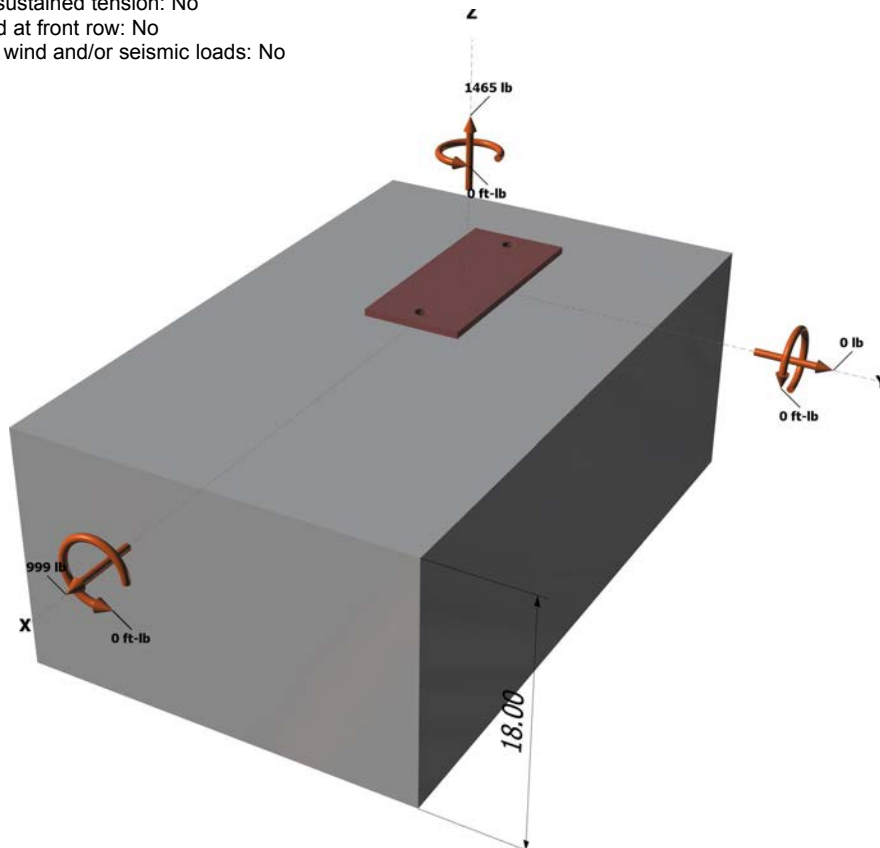
Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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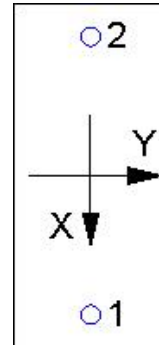
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Anchor Designer™
Software
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Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.