



Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	2000 mm	Height =	1900 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 30°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	16.49 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.73	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	85 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 11.34 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

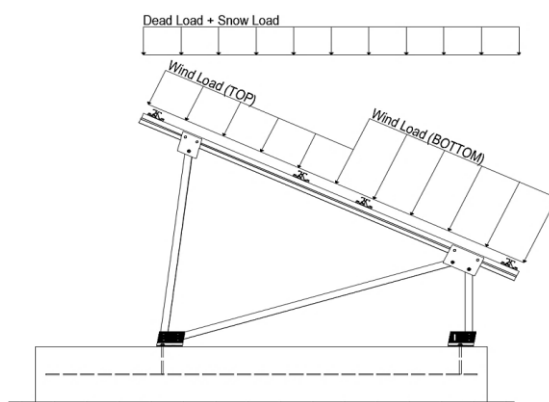
$C_{f+ TOP}$ =	1.150	(Pressure)
$C_{f+ BOTTOM}$ =	1.850	
$C_{f- TOP, OUTER PURLIN}$ =	-2.600	
$C_{f- TOP, INNER PURLIN}$ =	-2.000	(Suction)
$C_{f- BOTTOM}$ =	-1.100	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.07	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	120 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.708 k-ft
$M_z$ =	0.401 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>96%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	104.56 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.00 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.772 k
$M_{y \text{ allowable}}$ =	3.422 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>89%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

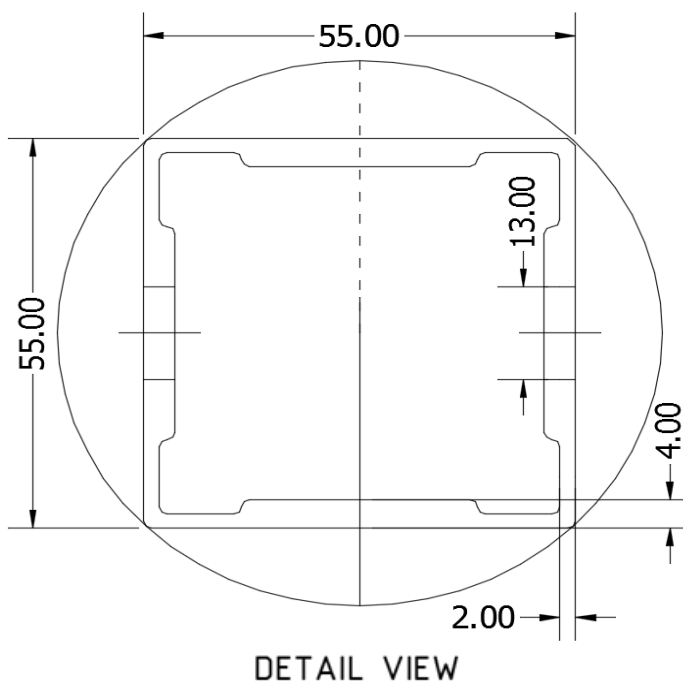
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.666 k-ft
$P_n$ =	0.163 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>48%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	98.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	6.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.093 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	6.000 k
Utilization =	<b>36%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.35 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.88 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.010 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.089 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.726 k
Utilization =	<b>36%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

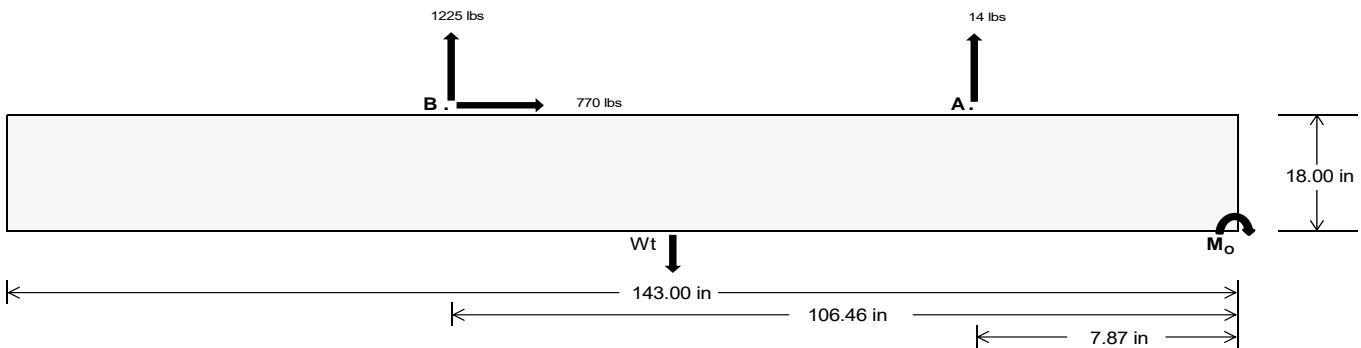
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

Maximum	Front	Rear
Tensile Load =	<b>74.01</b>	<b>5110.02</b> k
Compressive Load =	<b>3623.45</b>	<b>4385.46</b> k
Lateral Load =	<b>435.65</b>	<b>3204.06</b> k
Moment (Weak Axis) =	<b>0.87</b>	<b>0.34</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 144397.0$  in-lbs  
Resisting Force Required = 2019.54 lbs  
S.F. = 1.67  
Weight Required = 3365.90 lbs  
Minimum Width = 35 in  
Weight Provided = 7559.64 lbs

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.

### Sliding

Force = 769.92 lbs  
Friction = 0.4  
Weight Required = 1924.80 lbs  
Resisting Weight = 7559.64 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

### Cohesion

Sliding Force = 769.92 lbs  
Cohesion = 130 psf  
Area = 34.76 ft<sup>2</sup>  
Resisting = 3779.82 lbs  
Additional Weight Required = 0 lbs

Use a 143in long x 35in wide x 18in tall ballast foundation. Cohesion is OK.

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11.92 \text{ ft})(1.5 \text{ ft})(2.92 \text{ ft}) = 7560 \text{ lbs}$     35 in    36 in    37 in    38 in  
7560 lbs    7776 lbs    7992 lbs    8208 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in	35 in	36 in	37 in	38 in
$F_A$	1398 lbs	1398 lbs	1398 lbs	1398 lbs	1129 lbs	1129 lbs	1129 lbs	1129 lbs	1749 lbs	1749 lbs	1749 lbs	1749 lbs	-29 lbs	-29 lbs	-29 lbs	-29 lbs
$F_B$	1394 lbs	1394 lbs	1394 lbs	1394 lbs	1835 lbs	1835 lbs	1835 lbs	1835 lbs	2277 lbs	2277 lbs	2277 lbs	2277 lbs	-2450 lbs	-2450 lbs	-2450 lbs	-2450 lbs
$F_V$	207 lbs	207 lbs	207 lbs	207 lbs	1411 lbs	1411 lbs	1411 lbs	1411 lbs	1194 lbs	1194 lbs	1194 lbs	1194 lbs	-1540 lbs	-1540 lbs	-1540 lbs	-1540 lbs
$P_{total}$	10352 lbs	10568 lbs	10784 lbs	11000 lbs	10523 lbs	10739 lbs	10955 lbs	11171 lbs	11586 lbs	11802 lbs	12018 lbs	12234 lbs	2057 lbs	2187 lbs	2316 lbs	2446 lbs
$M$	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	3663 lbs-ft	2757 lbs-ft	2757 lbs-ft	2757 lbs-ft	2757 lbs-ft	4432 lbs-ft	4432 lbs-ft	4432 lbs-ft	4432 lbs-ft	4677 lbs-ft	4677 lbs-ft	4677 lbs-ft	4677 lbs-ft
$e$	0.35 ft	0.35 ft	0.34 ft	0.33 ft	0.26 ft	0.26 ft	0.25 ft	0.25 ft	0.38 ft	0.38 ft	0.37 ft	0.36 ft	2.27 ft	2.14 ft	2.02 ft	1.91 ft
$L/6$	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft	1.99 ft
$f_{min}$	244.8 psf	244.0 psf	243.3 psf	242.6 psf	262.8 psf	261.6 psf	260.4 psf	259.2 psf	269.1 psf	267.7 psf	266.4 psf	265.1 psf	0.0 psf	0.0 psf	0.0 psf	2.4 psf
$f_{max}$	350.9 psf	347.2 psf	343.7 psf	340.4 psf	342.7 psf	339.2 psf	335.9 psf	332.8 psf	397.5 psf	392.5 psf	387.8 psf	383.3 psf	127.6 psf	127.2 psf	127.1 psf	127.2 psf

Maximum Bearing Pressure = 398 psf  
Allowable Bearing Pressure = 1500 psf

Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

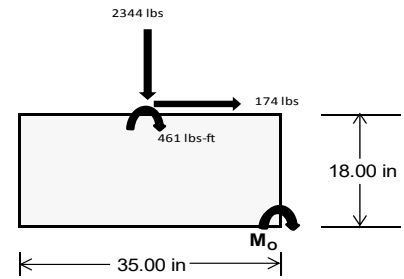
### Overturning Check

$M_o = 2695.9 \text{ ft-lbs}$   
 Resisting Force Required = 1848.62 lbs  
 S.F. = 1.67  
 Weight Required = 3081.03 lbs  
 Minimum Width = 35 in  
 Weight Provided = 7559.64 lbs

*A minimum 143in long x 35in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	35 in			35 in			35 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	350 lbs	720 lbs	226 lbs	899 lbs	2344 lbs	803 lbs	145 lbs	211 lbs	23 lbs
$F_h$	244 lbs	238 lbs	251 lbs	177 lbs	174 lbs	198 lbs	246 lbs	239 lbs	248 lbs
$P_{total}$	9708 lbs	10079 lbs	9585 lbs	9808 lbs	11253 lbs	9712 lbs	2882 lbs	2947 lbs	2760 lbs
$M$	969 lbs-ft	953 lbs-ft	990 lbs-ft	718 lbs-ft	722 lbs-ft	784 lbs-ft	969 lbs-ft	950 lbs-ft	977 lbs-ft
$e$	0.10 ft	0.09 ft	0.10 ft	0.07 ft	0.06 ft	0.08 ft	0.34 ft	0.32 ft	0.35 ft
$L/6$	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft	0.49 ft
$f_{min}$	222.0 psf	233.6 psf	217.2 psf	239.7 psf	281.0 psf	233.1 psf	25.5 psf	28.6 psf	21.6 psf
$f_{max}$	336.7 psf	346.4 psf	334.4 psf	324.7 psf	366.5 psf	325.8 psf	140.3 psf	141.0 psf	137.2 psf



Maximum Bearing Pressure = 366 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 143in long x 35in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 143in long x 26in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.516 k
Allowable Uplift =	1.214 k
Utilization =	<u>42%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.823 k
Allowable Uplift =	4.357 k
Utilization =	<u>42%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	2.787 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>38%</u>

#### Rear Strut

Maximum Axial Load =	3.432 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>46%</u>

#### Diagonal Strut

Maximum Axial Load =	2.173 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>29%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	60.93 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.219 in
	<u>0.996 ≤ 1.219, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 120 \text{ in}$$

$$J = 0.432$$

$$331.976$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 120$$

$$J = 0.432$$

$$211.117$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.6$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \text{ in} \\ J &= 1.08 \\ &= 179.85 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.0 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 104.56 \\ J &= 1.08 \\ &= 190.335 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 28.9 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.0 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.323 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi F_L = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi F_L = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 98.03 \text{ in}$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 98.03$$

$$J = 0.942$$

$$152.985$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi_b [Bc - 1.6Dc^* \sqrt{((LbSc)/((Cb^* \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.26776$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.89749$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 6.10803 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 6.11 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 6.29 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \text{ in} \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.35 \\ J &= 0.942 \\ &= 122.273 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi_b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi_b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.8125$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83375$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 8.88278 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 8.88 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 9.14 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-9.843	-9.843	0	0
2	M14	Y	-9.843	-9.843	0	0
3	M15	Y	-9.843	-9.843	0	0
4	M16	Y	-9.843	-9.843	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-5.454	-5.454	0	0
2	M14	Y	-5.454	-5.454	0	0
3	M15	Y	-5.454	-5.454	0	0
4	M16	Y	-5.454	-5.454	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-46.866	-46.866	0	0
2	M14	Y	-46.866	-46.866	0	0
3	M15	Y	-46.866	-46.866	0	0
4	M16	Y	-46.866	-46.866	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-42.8	-42.8	0	0
2	M14	y	-42.8	-42.8	0	0
3	M15	y	-68.853	-68.853	0	0
4	M16	y	-68.853	-68.853	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	96.766	96.766	0	0
2	M14	y	74.435	74.435	0	0
3	M15	y	40.939	40.939	0	0
4	M16	y	40.939	40.939	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	7.874	7.874	0	0
2	M14	Z	7.874	7.874	0	0
3	M15	Z	7.874	7.874	0	0
4	M16	Z	7.874	7.874	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMMax 72 Cell 2V 30° 85mph 30psf 10ft 7-05.r3d] Page 19



Company : Schletter, Inc.  
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 Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	108.179	1	700.014	1	-10.138	12	.004	14	.403	1	1.452	1
20			min	6.37	12	-933.428	3	-231.445	1	-.014	2	.015	12	-1.864	3
21		11	max	108.179	1	576.712	1	-7.819	12	.014	2	.173	1	.742	1
22			min	6.37	12	-767.426	3	-182.222	1	0	3	.005	12	-.919	3
23		12	max	108.179	1	453.41	1	-5.5	12	.014	2	.08	4	.17	1
24			min	6.37	12	-601.423	3	-132.998	1	0	3	-.004	3	-.159	3
25		13	max	108.179	1	330.109	1	-3.18	12	.014	2	.036	5	.417	3
26			min	6.37	12	-435.421	3	-83.775	1	0	3	-.122	1	-.265	1
27		14	max	108.179	1	206.807	1	-.861	12	.014	2	-.002	15	.809	3
28			min	6.37	12	-269.418	3	-41.785	4	0	3	-.188	1	-.564	1
29		15	max	108.179	1	83.505	1	14.672	1	.014	2	-.009	12	1.016	3
30			min	-2.122	5	-103.416	3	-29.662	5	0	3	-.199	1	-.725	1
31		16	max	108.179	1	62.587	3	63.896	1	.014	2	-.006	12	1.039	3
32			min	-15.233	5	-39.797	1	-26.132	5	0	3	-.155	1	-.749	1
33		17	max	108.179	1	228.589	3	113.119	1	.014	2	0	12	.877	3
34			min	-28.345	5	-163.099	1	-22.602	5	0	3	-.111	4	-.636	1
35		18	max	108.179	1	394.592	3	162.342	1	.014	2	.096	1	.531	3
36			min	-41.457	5	-286.401	1	-19.072	5	0	3	-.119	5	-.387	1
37		19	max	108.179	1	560.594	3	211.566	1	.014	2	.304	1	0	1
38			min	-54.568	5	-409.703	1	-15.542	5	0	3	-.138	5	0	3
39	M14	1	max	62.465	4	445.771	1	-11.088	12	.009	3	.354	1	0	4
40			min	3.212	12	-445.748	3	-219.134	1	-.012	1	.02	12	0	3
41		2	max	58.642	1	322.469	1	-8.768	12	.009	3	.234	4	.425	3
42			min	3.212	12	-319.445	3	-169.911	1	-.012	1	.009	12	-.427	1
43		3	max	58.642	1	199.167	1	-6.449	12	.009	3	.133	5	.71	3
44			min	3.212	12	-193.142	3	-120.687	1	-.012	1	-.023	1	-.717	1
45		4	max	58.642	1	75.865	1	-4.13	12	.009	3	.072	5	.854	3
46			min	3.212	12	-66.839	3	-71.464	1	-.012	1	-.13	1	-.869	1
47		5	max	58.642	1	59.464	3	-1.81	12	.009	3	.015	5	.858	3
48			min	-3.498	5	-47.437	1	-55.532	4	-.012	1	-.182	1	-.885	1
49		6	max	58.642	1	185.767	3	26.983	1	.009	3	-.009	12	.722	3
50			min	-16.61	5	-170.739	1	-45.619	5	-.012	1	-.18	1	-.764	1
51		7	max	58.642	1	312.07	3	76.207	1	.009	3	-.007	12	.446	3
52			min	-29.721	5	-294.041	1	-42.089	5	-.012	1	-.122	1	-.506	1
53		8	max	58.642	1	438.373	3	125.43	1	.009	3	0	10	.029	3
54			min	-42.833	5	-417.343	1	-38.558	5	-.012	1	-.137	4	-.118	2
55		9	max	58.642	1	564.676	3	174.653	1	.009	3	.157	1	.422	1
56			min	-55.945	5	-540.644	1	-35.028	5	-.012	1	-.172	5	-.529	3
57		10	max	90.43	4	663.946	1	-9.787	12	.009	3	.378	1	1.091	1
58			min	3.212	12	-690.979	3	-223.877	1	-.012	1	.014	12	-1.226	3
59		11	max	77.319	4	540.644	1	-7.467	12	.012	1	.234	4	.422	1
60			min	3.212	12	-564.676	3	-174.653	1	-.009	3	.004	12	-.529	3
61		12	max	64.207	4	417.343	1	-5.148	12	.012	1	.13	4	.029	3
62			min	3.212	12	-438.373	3	-125.43	1	-.009	3	-.01	1	-.118	2
63		13	max	58.642	1	294.041	1	-2.828	12	.012	1	.068	5	.446	3
64			min	3.212	12	-312.07	3	-76.207	1	-.009	3	-.122	1	-.506	1
65		14	max	58.642	1	170.739	1	-.509	12	.012	1	.011	5	.722	3
66			min	3.212	12	-185.767	3	-56.692	4	-.009	3	-.18	1	-.764	1
67		15	max	58.642	1	47.437	1	22.24	1	.012	1	-.008	12	.858	3
68			min	3.212	12	-59.464	3	-45.895	5	-.009	3	-.182	1	-.885	1
69		16	max	58.642	1	66.839	3	71.464	1	.012	1	-.005	12	.854	3
70			min	-2.043	5	-75.865	1	-42.365	5	-.009	3	-.13	1	-.869	1
71		17	max	58.642	1	193.142	3	120.687	1	.012	1	.002	3	.71	3
72			min	-15.155	5	-199.167	1	-38.834	5	-.009	3	-.144	4	-.717	1
73		18	max	58.642	1	319.445	3	169.911	1	.012	1	.138	1	.425	3
74			min	-28.266	5	-322.469	1	-35.304	5	-.009	3	-.177	5	-.427	1
75		19	max	58.642	1	445.748	3	219.134	1	.012	1	.354	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-41.378	5	-445.771	1	-31.774	5	-.009	3	-.214	5	0	3
77	M15	1	max	101.605	5	562.59	2	-11.02	12	.013	1	.424	4	0	2
78			min	-62.925	1	-245.109	3	-219.056	1	-.008	3	.02	12	0	3
79		2	max	88.493	5	404.534	2	-8.7	12	.013	1	.289	4	.235	3
80			min	-62.925	1	-178.354	3	-169.832	1	-.008	3	.009	12	-.537	2
81		3	max	75.382	5	246.478	2	-6.381	12	.013	1	.172	5	.396	3
82			min	-62.925	1	-111.6	3	-120.609	1	-.008	3	-.024	1	-.899	2
83		4	max	62.27	5	88.421	2	-4.062	12	.013	1	.096	5	.483	3
84			min	-62.925	1	-44.845	3	-84.265	4	-.008	3	-.13	1	-1.085	2
85		5	max	49.158	5	21.909	3	-1.742	12	.013	1	.024	5	.496	3
86			min	-62.925	1	-69.635	2	-69.35	4	-.008	3	-.182	1	-1.095	2
87		6	max	36.047	5	88.664	3	27.061	1	.013	1	-.009	12	.435	3
88			min	-62.925	1	-227.691	2	-59.374	5	-.008	3	-.18	1	-.93	2
89		7	max	22.935	5	155.418	3	76.285	1	.013	1	-.007	12	.299	3
90			min	-62.925	1	-385.748	2	-55.843	5	-.008	3	-.138	4	-.589	2
91		8	max	9.823	5	222.173	3	125.508	1	.013	1	0	10	.089	3
92			min	-62.925	1	-543.804	2	-52.313	5	-.008	3	-.174	4	-.088	1
93		9	max	-2.071	15	288.927	3	174.732	1	.013	1	.157	1	.619	2
94			min	-62.925	1	-701.86	2	-48.783	5	-.008	3	-.224	5	-.195	3
95		10	max	-3.769	12	859.917	2	-9.855	12	.013	1	.423	4	1.487	2
96			min	-62.925	1	-355.682	3	-223.955	1	-.008	3	.014	12	-.553	3
97		11	max	-3.769	12	701.86	2	-7.535	12	.008	3	.287	4	.619	2
98			min	-62.925	1	-288.927	3	-174.732	1	-.013	1	.005	12	-.195	3
99		12	max	-3.769	12	543.804	2	-5.216	12	.008	3	.167	4	.089	3
100			min	-62.925	1	-222.173	3	-125.508	1	-.013	1	-.01	1	-.088	1
101		13	max	-3.769	12	385.748	2	-2.896	12	.008	3	.09	5	.299	3
102			min	-62.925	1	-155.418	3	-85.481	4	-.013	1	-.122	1	-.589	2
103		14	max	-3.769	12	227.691	2	-.577	12	.008	3	.018	5	.435	3
104			min	-62.925	1	-88.664	3	-70.566	4	-.013	1	-.18	1	-.93	2
105		15	max	-3.769	12	69.635	2	22.162	1	.008	3	-.008	12	.496	3
106			min	-74.692	4	-21.909	3	-59.656	5	-.013	1	-.182	1	-1.095	2
107		16	max	-3.769	12	44.845	3	71.385	1	.008	3	-.005	12	.483	3
108			min	-87.804	4	-88.421	2	-56.126	5	-.013	1	-.147	4	-1.085	2
109		17	max	-3.769	12	111.6	3	120.609	1	.008	3	.001	3	.396	3
110			min	-100.915	4	-246.478	2	-52.596	5	-.013	1	-.184	4	-.899	2
111		18	max	-3.769	12	178.354	3	169.832	1	.008	3	.138	1	.235	3
112			min	-114.027	4	-404.534	2	-49.066	5	-.013	1	-.232	5	-.537	2
113		19	max	-3.769	12	245.109	3	219.056	1	.008	3	.354	1	0	2
114			min	-127.139	4	-562.59	2	-45.536	5	-.013	1	-.284	5	0	5
115	M16	1	max	96.069	5	528.211	2	-10.536	12	.011	1	.317	4	0	2
116			min	-121.738	1	-219.944	3	-212.044	1	-.011	3	.017	12	0	3
117		2	max	82.957	5	370.154	2	-8.217	12	.011	1	.206	4	.207	3
118			min	-121.738	1	-153.19	3	-162.82	1	-.011	3	.007	12	-.499	2
119		3	max	69.846	5	212.098	2	-5.898	12	.011	1	.122	5	.34	3
120			min	-121.738	1	-86.435	3	-113.597	1	-.011	3	-.055	1	-.823	2
121		4	max	56.734	5	54.042	2	-3.578	12	.011	1	.068	5	.399	3
122			min	-121.738	1	-19.681	3	-64.373	1	-.011	3	-.154	1	-.97	2
123		5	max	43.622	5	47.074	3	-1.259	12	.011	1	.018	5	.384	3
124			min	-121.738	1	-104.015	2	-47.686	4	-.011	3	-.198	1	-.943	2
125		6	max	30.511	5	113.828	3	34.074	1	.011	1	-.009	12	.295	3
126			min	-121.738	1	-262.071	2	-39.726	5	-.011	3	-.188	1	-.739	2
127		7	max	17.399	5	180.583	3	83.297	1	.011	1	-.007	12	.131	3
128			min	-121.738	1	-420.127	2	-36.196	5	-.011	3	-.122	1	-.36	2
129		8	max	4.287	5	247.337	3	132.52	1	.011	1	0	10	.194	2
130			min	-121.738	1	-578.183	2	-32.665	5	-.011	3	-.112	4	-.107	3
131		9	max	-5.809	15	314.092	3	181.744	1	.011	1	.172	1	.925	2
132			min	-121.738	1	-736.24	2	-29.135	5	-.011	3	-.143	5	-.418	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-6.816	12	894.296	2	-10.338	12	.011	1	.401	1	1.83	2
134		min	-121.738	1	-380.846	3	-230.967	1	-.011	3	.016	12	-.805	3
135	11	max	-5.497	15	736.24	2	-8.019	12	.011	3	.211	4	.925	2
136		min	-121.738	1	-314.092	3	-181.744	1	-.011	1	.006	12	-.418	3
137	12	max	-6.816	12	578.183	2	-5.699	12	.011	3	.11	4	.194	2
138		min	-121.738	1	-247.337	3	-132.52	1	-.011	1	-.003	3	-.107	3
139	13	max	-6.816	12	420.127	2	-3.38	12	.011	3	.053	5	.131	3
140		min	-121.738	1	-180.583	3	-83.297	1	-.011	1	-.122	1	-.36	2
141	14	max	-6.816	12	262.071	2	-1.06	12	.011	3	.002	5	.295	3
142		min	-121.738	1	-113.828	3	-53.224	4	-.011	1	-.188	1	-.739	2
143	15	max	-6.816	12	104.015	2	15.15	1	.011	3	-.009	12	.384	3
144		min	-121.738	1	-47.074	3	-41.052	5	-.011	1	-.198	1	-.943	2
145	16	max	-6.816	12	19.681	3	64.373	1	.011	3	-.006	12	.399	3
146		min	-121.738	1	-54.042	2	-37.522	5	-.011	1	-.154	1	-.97	2
147	17	max	-6.816	12	86.435	3	113.597	1	.011	3	-.001	12	.34	3
148		min	-121.738	1	-212.098	2	-33.992	5	-.011	1	-.144	4	-.823	2
149	18	max	-6.816	12	153.19	3	162.82	1	.011	3	.099	1	.207	3
150		min	-129.875	4	-370.154	2	-30.462	5	-.011	1	-.165	5	-.499	2
151	19	max	-6.816	12	219.944	3	212.044	1	.011	3	.307	1	0	2
152		min	-142.987	4	-528.211	2	-26.931	5	-.011	1	-.197	5	0	5
153	M2	1	max	972.702	1	2.056	4	.605	1	0	12	0	3	0
154		min	-1060.602	3	.498	15	-37.161	4	0	4	0	1	0	1
155	2	max	973.232	1	1.985	4	.605	1	0	12	0	1	0	15
156		min	-1060.205	3	.481	15	-37.623	4	0	4	-.013	4	0	4
157	3	max	973.761	1	1.914	4	.605	1	0	12	0	1	0	15
158		min	-1059.808	3	.464	15	-38.084	4	0	4	-.027	4	-.001	4
159	4	max	974.29	1	1.843	4	.605	1	0	12	0	1	0	15
160		min	-1059.411	3	.448	15	-38.545	4	0	4	-.041	4	-.002	4
161	5	max	974.819	1	1.772	4	.605	1	0	12	0	1	0	15
162		min	-1059.014	3	.431	15	-39.006	4	0	4	-.055	4	-.003	4
163	6	max	975.349	1	1.701	4	.605	1	0	12	.001	1	0	15
164		min	-1058.617	3	.414	15	-39.467	4	0	4	-.069	4	-.003	4
165	7	max	975.878	1	1.63	4	.605	1	0	12	.001	1	0	15
166		min	-1058.221	3	.398	15	-39.929	4	0	4	-.083	4	-.004	4
167	8	max	976.407	1	1.559	4	.605	1	0	12	.002	1	-.001	15
168		min	-1057.824	3	.381	15	-40.39	4	0	4	-.097	4	-.005	4
169	9	max	976.937	1	1.488	4	.605	1	0	12	.002	1	-.001	15
170		min	-1057.427	3	.364	15	-40.851	4	0	4	-.112	4	-.005	4
171	10	max	977.466	1	1.417	4	.605	1	0	12	.002	1	-.001	15
172		min	-1057.03	3	.348	15	-41.312	4	0	4	-.127	4	-.006	4
173	11	max	977.995	1	1.345	4	.605	1	0	12	.002	1	-.001	15
174		min	-1056.633	3	.331	15	-41.774	4	0	4	-.142	4	-.006	4
175	12	max	978.524	1	1.274	4	.605	1	0	12	.002	1	-.002	15
176		min	-1056.236	3	.314	15	-42.235	4	0	4	-.157	4	-.007	4
177	13	max	979.054	1	1.203	4	.605	1	0	12	.003	1	-.002	15
178		min	-1055.839	3	.297	15	-42.696	4	0	4	-.172	4	-.007	4
179	14	max	979.583	1	1.132	4	.605	1	0	12	.003	1	-.002	15
180		min	-1055.442	3	.281	15	-43.157	4	0	4	-.187	4	-.007	4
181	15	max	980.112	1	1.061	4	.605	1	0	12	.003	1	-.002	15
182		min	-1055.045	3	.263	12	-43.618	4	0	4	-.203	4	-.008	4
183	16	max	980.642	1	.99	4	.605	1	0	12	.003	1	-.002	15
184		min	-1054.648	3	.235	12	-44.08	4	0	4	-.219	4	-.008	4
185	17	max	981.171	1	.919	4	.605	1	0	12	.003	1	-.002	15
186		min	-1054.251	3	.208	12	-44.541	4	0	4	-.234	4	-.009	4
187	18	max	981.7	1	.848	4	.605	1	0	12	.004	1	-.002	15
188		min	-1053.854	3	.18	12	-45.002	4	0	4	-.251	4	-.009	4
189	19	max	982.23	1	.777	4	.605	1	0	12	.004	1	-.002	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190			min	-1053.457	3	.152	12	-45.463	4	0	4	-.267	4	-.009	4
191	M3	1	max	546.548	2	8.9	4	1.931	4	0	12	0	1	.009	4
192			min	-702.761	3	2.103	15	.026	12	0	4	-.029	4	.002	15
193		2	max	546.378	2	8.031	4	2.536	4	0	12	0	1	.005	4
194			min	-702.888	3	1.899	15	.026	12	0	4	-.028	4	.001	12
195		3	max	546.208	2	7.162	4	3.141	4	0	12	0	1	.002	2
196			min	-703.016	3	1.695	15	.026	12	0	4	-.026	4	0	3
197		4	max	546.037	2	6.293	4	3.746	4	0	12	.001	1	0	15
198			min	-703.144	3	1.49	15	.026	12	0	4	-.025	4	-.002	3
199		5	max	545.867	2	5.424	4	4.351	4	0	12	.001	1	0	15
200			min	-703.272	3	1.286	15	.026	12	0	4	-.023	4	-.004	6
201		6	max	545.697	2	4.555	4	4.956	4	0	12	.002	1	-.002	15
202			min	-703.4	3	1.082	15	.026	12	0	4	-.021	5	-.007	6
203		7	max	545.526	2	3.686	4	5.561	4	0	12	.002	1	-.002	15
204			min	-703.527	3	.878	15	.026	12	0	4	-.018	5	-.009	6
205		8	max	545.356	2	2.818	4	6.166	4	0	12	.002	1	-.002	15
206			min	-703.655	3	.673	15	.026	12	0	4	-.016	5	-.01	6
207		9	max	545.185	2	1.949	4	6.771	4	0	12	.002	1	-.003	15
208			min	-703.783	3	.469	15	.026	12	0	4	-.013	5	-.011	6
209		10	max	545.015	2	1.08	4	7.377	4	0	12	.002	1	-.003	15
210			min	-703.911	3	.265	15	.026	12	0	4	-.009	5	-.012	6
211		11	max	544.845	2	.266	2	7.982	4	0	12	.003	1	-.003	15
212			min	-704.038	3	-.059	3	.026	12	0	4	-.006	5	-.012	6
213		12	max	544.674	2	-.144	15	8.587	4	0	12	.003	1	-.003	15
214			min	-704.166	3	-.659	6	.026	12	0	4	-.002	5	-.012	6
215		13	max	544.504	2	-.348	15	9.192	4	0	12	.003	1	-.003	15
216			min	-704.294	3	-1.528	6	.026	12	0	4	0	12	-.012	6
217		14	max	544.334	2	-.552	15	9.797	4	0	12	.007	4	-.003	15
218			min	-704.422	3	-2.397	6	.026	12	0	4	0	12	-.011	6
219		15	max	544.163	2	-.756	15	10.402	4	0	12	.012	4	-.002	15
220			min	-704.549	3	-3.266	6	.026	12	0	4	0	12	-.009	6
221		16	max	543.993	2	-.961	15	11.007	4	0	12	.017	4	-.002	15
222			min	-704.677	3	-4.135	6	.026	12	0	4	0	12	-.008	6
223		17	max	543.823	2	-1.165	15	11.612	4	0	12	.022	4	-.001	15
224			min	-704.805	3	-5.003	6	.026	12	0	4	0	12	-.006	6
225		18	max	543.652	2	-1.369	15	12.217	4	0	12	.028	4	0	15
226			min	-704.933	3	-5.872	6	.026	12	0	4	0	12	-.003	6
227		19	max	543.482	2	-1.573	15	12.822	4	0	12	.034	4	0	1
228			min	-705.06	3	-6.741	6	.026	12	0	4	0	12	0	1
229	M4	1	max	1095.495	1	0	1	-1.067	12	0	1	.027	4	0	1
230			min	-57.435	5	0	1	-334.056	4	0	1	0	12	0	1
231		2	max	1095.665	1	0	1	-1.067	12	0	1	.001	1	0	1
232			min	-57.356	5	0	1	-334.204	4	0	1	-.011	4	0	1
233		3	max	1095.835	1	0	1	-1.067	12	0	1	0	12	0	1
234			min	-57.276	5	0	1	-334.352	4	0	1	-.05	4	0	1
235		4	max	1096.006	1	0	1	-1.067	12	0	1	0	12	0	1
236			min	-57.197	5	0	1	-334.499	4	0	1	-.088	4	0	1
237		5	max	1096.176	1	0	1	-1.067	12	0	1	0	12	0	1
238			min	-57.117	5	0	1	-334.647	4	0	1	-.126	4	0	1
239		6	max	1096.346	1	0	1	-1.067	12	0	1	0	12	0	1
240			min	-57.038	5	0	1	-334.795	4	0	1	-.165	4	0	1
241		7	max	1096.517	1	0	1	-1.067	12	0	1	0	12	0	1
242			min	-56.958	5	0	1	-334.942	4	0	1	-.203	4	0	1
243		8	max	1096.687	1	0	1	-1.067	12	0	1	0	12	0	1
244			min	-56.879	5	0	1	-335.09	4	0	1	-.242	4	0	1
245		9	max	1096.858	1	0	1	-1.067	12	0	1	0	12	0	1
246			min	-56.799	5	0	1	-335.238	4	0	1	-.28	4	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247	10	max	1097.028	1	0	1	-1.067	12	0	1	0	12	0	1
248		min	-56.72	5	0	1	-335.385	4	0	1	-.319	4	0	1
249	11	max	1097.198	1	0	1	-1.067	12	0	1	-.001	12	0	1
250		min	-56.64	5	0	1	-335.533	4	0	1	-.357	4	0	1
251	12	max	1097.369	1	0	1	-1.067	12	0	1	-.001	12	0	1
252		min	-56.561	5	0	1	-335.68	4	0	1	-.396	4	0	1
253	13	max	1097.539	1	0	1	-1.067	12	0	1	-.001	12	0	1
254		min	-56.481	5	0	1	-335.828	4	0	1	-.434	4	0	1
255	14	max	1097.709	1	0	1	-1.067	12	0	1	-.001	12	0	1
256		min	-56.402	5	0	1	-335.976	4	0	1	-.473	4	0	1
257	15	max	1097.88	1	0	1	-1.067	12	0	1	-.002	12	0	1
258		min	-56.322	5	0	1	-336.123	4	0	1	-.511	4	0	1
259	16	max	1098.05	1	0	1	-1.067	12	0	1	-.002	12	0	1
260		min	-56.243	5	0	1	-336.271	4	0	1	-.55	4	0	1
261	17	max	1098.22	1	0	1	-1.067	12	0	1	-.002	12	0	1
262		min	-56.163	5	0	1	-336.419	4	0	1	-.589	4	0	1
263	18	max	1098.391	1	0	1	-1.067	12	0	1	-.002	12	0	1
264		min	-56.084	5	0	1	-336.566	4	0	1	-.627	4	0	1
265	19	max	1098.561	1	0	1	-1.067	12	0	1	-.002	12	0	1
266		min	-56.004	5	0	1	-336.714	4	0	1	-.666	4	0	1
267	M6	1	max	3079.301	1	2.149	2	0	1	0	0	4	0	1
268		min	-3431.658	3	.354	12	-37.604	4	0	4	0	1	0	1
269	2	max	3079.83	1	2.094	2	0	1	0	1	0	1	0	12
270		min	-3431.261	3	.327	12	-38.065	4	0	4	-.014	4	0	2
271	3	max	3080.359	1	2.038	2	0	1	0	1	0	1	0	12
272		min	-3430.864	3	.299	12	-38.526	4	0	4	-.027	4	-.002	2
273	4	max	3080.889	1	1.983	2	0	1	0	1	0	1	0	12
274		min	-3430.467	3	.271	12	-38.987	4	0	4	-.041	4	-.002	2
275	5	max	3081.418	1	1.928	2	0	1	0	1	0	1	0	12
276		min	-3430.07	3	.244	12	-39.449	4	0	4	-.055	4	-.003	2
277	6	max	3081.947	1	1.872	2	0	1	0	1	0	1	0	12
278		min	-3429.673	3	.216	12	-39.91	4	0	4	-.07	4	-.004	2
279	7	max	3082.476	1	1.817	2	0	1	0	1	0	1	0	12
280		min	-3429.276	3	.188	12	-40.371	4	0	4	-.084	4	-.004	2
281	8	max	3083.006	1	1.761	2	0	1	0	1	0	1	0	12
282		min	-3428.879	3	.161	12	-40.832	4	0	4	-.098	4	-.005	2
283	9	max	3083.535	1	1.706	2	0	1	0	1	0	1	0	12
284		min	-3428.482	3	.133	12	-41.293	4	0	4	-.113	4	-.006	2
285	10	max	3084.064	1	1.651	2	0	1	0	1	0	1	0	12
286		min	-3428.085	3	.105	12	-41.755	4	0	4	-.128	4	-.006	2
287	11	max	3084.594	1	1.595	2	0	1	0	1	0	1	0	12
288		min	-3427.688	3	.073	3	-42.216	4	0	4	-.143	4	-.007	2
289	12	max	3085.123	1	1.54	2	0	1	0	1	0	1	0	12
290		min	-3427.291	3	.031	3	-42.677	4	0	4	-.158	4	-.007	2
291	13	max	3085.652	1	1.485	2	0	1	0	1	0	1	0	12
292		min	-3426.894	3	-.01	3	-43.138	4	0	4	-.174	4	-.008	2
293	14	max	3086.181	1	1.429	2	0	1	0	1	0	1	0	12
294		min	-3426.497	3	-.052	3	-43.6	4	0	4	-.189	4	-.008	2
295	15	max	3086.711	1	1.374	2	0	1	0	1	0	1	0	12
296		min	-3426.1	3	-.093	3	-44.061	4	0	4	-.205	4	-.009	2
297	16	max	3087.24	1	1.319	2	0	1	0	1	0	1	0	12
298		min	-3425.703	3	-.135	3	-44.522	4	0	4	-.221	4	-.009	2
299	17	max	3087.769	1	1.263	2	0	1	0	1	0	1	0	12
300		min	-3425.306	3	-.176	3	-44.983	4	0	4	-.237	4	-.01	2
301	18	max	3088.299	1	1.208	2	0	1	0	1	0	1	0	12
302		min	-3424.909	3	-.218	3	-45.444	4	0	4	-.253	4	-.01	2
303	19	max	3088.828	1	1.153	2	0	1	0	1	0	1	0	12



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304		min	-3424.512	3	-.259	3	-45.906	4	0	4	-.27	4	-.011	2
305	M7	1	max	2093.011	2	8.909	6	1.399	4	0	1	0	.011	2
306		min	-2170.883	3	2.091	15	0	1	0	4	-.029	4	0	12
307		2	max	2092.841	2	8.04	6	2.004	4	0	1	0	.007	2
308		min	-2171.011	3	1.887	15	0	1	0	4	-.028	4	-.001	3
309		3	max	2092.671	2	7.171	6	2.609	4	0	1	0	.004	2
310		min	-2171.138	3	1.683	15	0	1	0	4	-.027	4	-.003	3
311		4	max	2092.5	2	6.302	6	3.215	4	0	1	0	.002	2
312		min	-2171.266	3	1.479	15	0	1	0	4	-.026	4	-.005	3
313		5	max	2092.33	2	5.433	6	3.82	4	0	1	0	0	2
314		min	-2171.394	3	1.274	15	0	1	0	4	-.024	4	-.006	3
315		6	max	2092.159	2	4.564	6	4.425	4	0	1	0	1	15
316		min	-2171.522	3	1.07	15	0	1	0	4	-.022	4	-.007	3
317		7	max	2091.989	2	3.695	6	5.03	4	0	1	0	1	15
318		min	-2171.65	3	.866	15	0	1	0	4	-.02	4	-.009	4
319		8	max	2091.819	2	2.826	6	5.635	4	0	1	0	1	15
320		min	-2171.777	3	.662	15	0	1	0	4	-.017	4	-.01	4
321		9	max	2091.648	2	1.957	6	6.24	4	0	1	0	1	15
322		min	-2171.905	3	.419	12	0	1	0	4	-.015	4	-.011	4
323		10	max	2091.478	2	1.258	2	6.845	4	0	1	0	1	15
324		min	-2172.033	3	.08	12	0	1	0	4	-.011	4	-.012	4
325		11	max	2091.308	2	.581	2	7.45	4	0	1	0	1	15
326		min	-2172.161	3	-.421	3	0	1	0	4	-.008	4	-.012	4
327		12	max	2091.137	2	-.096	2	8.055	4	0	1	0	1	15
328		min	-2172.288	3	-.929	3	0	1	0	4	-.004	4	-.012	4
329		13	max	2090.967	2	-.359	15	8.66	4	0	1	0	1	15
330		min	-2172.416	3	-1.518	4	0	1	0	4	0	4	-.012	4
331		14	max	2090.797	2	-.564	15	9.265	4	0	1	.004	4	15
332		min	-2172.544	3	-2.387	4	0	1	0	4	0	1	-.011	4
333		15	max	2090.626	2	-.768	15	9.87	4	0	1	.008	4	15
334		min	-2172.672	3	-3.256	4	0	1	0	4	0	1	-.009	4
335		16	max	2090.456	2	-.972	15	10.475	4	0	1	.013	4	15
336		min	-2172.799	3	-4.125	4	0	1	0	4	0	1	-.008	4
337		17	max	2090.286	2	-1.176	15	11.08	4	0	1	.018	4	15
338		min	-2172.927	3	-4.994	4	0	1	0	4	0	1	-.006	4
339		18	max	2090.115	2	-1.381	15	11.686	4	0	1	.023	4	15
340		min	-2173.055	3	-5.863	4	0	1	0	4	0	1	-.003	4
341		19	max	2089.945	2	-1.585	15	12.291	4	0	1	.029	4	1
342		min	-2173.183	3	-6.732	4	0	1	0	4	0	1	0	1
343	M8	1	max	2784.202	1	0	1	0	1	0	1	.023	4	1
344		min	-59.226	3	0	1	-317.409	4	0	1	0	1	0	1
345		2	max	2784.373	1	0	1	0	1	0	1	0	1	1
346		min	-59.099	3	0	1	-317.556	4	0	1	-.013	4	0	1
347		3	max	2784.543	1	0	1	0	1	0	1	0	1	1
348		min	-58.971	3	0	1	-317.704	4	0	1	-.05	4	0	1
349		4	max	2784.713	1	0	1	0	1	0	1	0	1	1
350		min	-58.843	3	0	1	-317.852	4	0	1	-.086	4	0	1
351		5	max	2784.884	1	0	1	0	1	0	1	0	1	1
352		min	-58.715	3	0	1	-317.999	4	0	1	-.123	4	0	1
353		6	max	2785.054	1	0	1	0	1	0	1	0	1	1
354		min	-58.588	3	0	1	-318.147	4	0	1	-.159	4	0	1
355		7	max	2785.224	1	0	1	0	1	0	1	0	1	1
356		min	-58.46	3	0	1	-318.295	4	0	1	-.196	4	0	1
357		8	max	2785.395	1	0	1	0	1	0	1	0	1	1
358		min	-58.332	3	0	1	-318.442	4	0	1	-.232	4	0	1
359		9	max	2785.565	1	0	1	0	1	0	1	0	1	1
360		min	-58.204	3	0	1	-318.59	4	0	1	-.269	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	2785.735	1	0	1	0	1	0	1	0	1	0	1
362			min	-58.077	3	0	1	-318.738	4	0	1	-.305	4	0	1
363		11	max	2785.906	1	0	1	0	1	0	1	0	1	0	1
364			min	-57.949	3	0	1	-318.885	4	0	1	-.342	4	0	1
365		12	max	2786.076	1	0	1	0	1	0	1	0	1	0	1
366			min	-57.821	3	0	1	-319.033	4	0	1	-.379	4	0	1
367		13	max	2786.247	1	0	1	0	1	0	1	0	1	0	1
368			min	-57.693	3	0	1	-319.18	4	0	1	-.415	4	0	1
369		14	max	2786.417	1	0	1	0	1	0	1	0	1	0	1
370			min	-57.566	3	0	1	-319.328	4	0	1	-.452	4	0	1
371		15	max	2786.587	1	0	1	0	1	0	1	0	1	0	1
372			min	-57.438	3	0	1	-319.476	4	0	1	-.489	4	0	1
373		16	max	2786.758	1	0	1	0	1	0	1	0	1	0	1
374			min	-57.31	3	0	1	-319.623	4	0	1	-.525	4	0	1
375		17	max	2786.928	1	0	1	0	1	0	1	0	1	0	1
376			min	-57.182	3	0	1	-319.771	4	0	1	-.562	4	0	1
377		18	max	2787.098	1	0	1	0	1	0	1	0	1	0	1
378			min	-57.055	3	0	1	-319.919	4	0	1	-.599	4	0	1
379		19	max	2787.269	1	0	1	0	1	0	1	0	1	0	1
380			min	-56.927	3	0	1	-320.066	4	0	1	-.635	4	0	1
381	M10	1	max	972.702	1	1.99	6	-.034	12	0	1	0	4	0	1
382			min	-1060.602	3	.453	15	-37.53	4	0	5	0	3	0	1
383		2	max	973.232	1	1.919	6	-.034	12	0	1	0	10	0	15
384			min	-1060.205	3	.437	15	-37.992	4	0	5	-.014	4	0	6
385		3	max	973.761	1	1.848	6	-.034	12	0	1	0	12	0	15
386			min	-1059.808	3	.42	15	-38.453	4	0	5	-.027	4	-.001	6
387		4	max	974.29	1	1.777	6	-.034	12	0	1	0	12	0	15
388			min	-1059.411	3	.403	15	-38.914	4	0	5	-.041	4	-.002	6
389		5	max	974.819	1	1.706	6	-.034	12	0	1	0	12	0	15
390			min	-1059.014	3	.387	15	-39.375	4	0	5	-.055	4	-.003	6
391		6	max	975.349	1	1.634	6	-.034	12	0	1	0	12	0	15
392			min	-1058.617	3	.37	15	-39.836	4	0	5	-.069	4	-.003	6
393		7	max	975.878	1	1.563	6	-.034	12	0	1	0	12	0	15
394			min	-1058.221	3	.353	15	-40.298	4	0	5	-.084	4	-.004	6
395		8	max	976.407	1	1.492	6	-.034	12	0	1	0	12	0	15
396			min	-1057.824	3	.337	15	-40.759	4	0	5	-.098	4	-.004	6
397		9	max	976.937	1	1.421	6	-.034	12	0	1	0	12	-.001	15
398			min	-1057.427	3	.32	15	-41.22	4	0	5	-.113	4	-.005	6
399		10	max	977.466	1	1.35	6	-.034	12	0	1	0	12	-.001	15
400			min	-1057.03	3	.303	15	-41.681	4	0	5	-.128	4	-.005	6
401		11	max	977.995	1	1.279	6	-.034	12	0	1	0	12	-.001	15
402			min	-1056.633	3	.286	15	-42.143	4	0	5	-.143	4	-.006	6
403		12	max	978.524	1	1.208	6	-.034	12	0	1	0	12	-.001	15
404			min	-1056.236	3	.27	15	-42.604	4	0	5	-.158	4	-.006	6
405		13	max	979.054	1	1.137	6	-.034	12	0	1	0	12	-.002	15
406			min	-1055.839	3	.253	15	-43.065	4	0	5	-.173	4	-.007	6
407		14	max	979.583	1	1.066	6	-.034	12	0	1	0	12	-.002	15
408			min	-1055.442	3	.236	15	-43.526	4	0	5	-.189	4	-.007	6
409		15	max	980.112	1	.995	6	-.034	12	0	1	0	12	-.002	15
410			min	-1055.045	3	.22	15	-43.987	4	0	5	-.205	4	-.007	6
411		16	max	980.642	1	.924	6	-.034	12	0	1	0	12	-.002	15
412			min	-1054.648	3	.203	15	-44.449	4	0	5	-.221	4	-.008	6
413		17	max	981.171	1	.853	6	-.034	12	0	1	0	12	-.002	15
414			min	-1054.251	3	.186	15	-44.91	4	0	5	-.237	4	-.008	6
415		18	max	981.7	1	.794	2	-.034	12	0	1	0	12	-.002	15
416			min	-1053.854	3	.17	15	-45.371	4	0	5	-.253	4	-.008	6
417		19	max	982.23	1	.739	2	-.034	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1053.457	3	.152	12	-45.832	4	0	5	-.269	4	-.009	6
419	M11	1	max	546.548	2	8.849	6	1.645	5	0	1	0	12	.009	6
420			min	-702.761	3	2.069	15	-.472	1	0	4	-.029	4	.002	15
421		2	max	546.378	2	7.98	6	2.251	5	0	1	0	12	.005	2
422			min	-702.888	3	1.865	15	-.472	1	0	4	-.028	4	.001	15
423		3	max	546.208	2	7.112	6	2.856	5	0	1	0	12	.002	2
424			min	-703.016	3	1.661	15	-.472	1	0	4	-.027	4	0	3
425		4	max	546.037	2	6.243	6	3.461	5	0	1	0	12	0	2
426			min	-703.144	3	1.457	15	-.472	1	0	4	-.025	4	-.002	3
427		5	max	545.867	2	5.374	6	4.066	5	0	1	0	12	-.001	15
428			min	-703.272	3	1.252	15	-.472	1	0	4	-.023	4	-.005	4
429		6	max	545.697	2	4.505	6	4.671	5	0	1	0	12	-.002	15
430			min	-703.4	3	1.048	15	-.472	1	0	4	-.021	4	-.007	4
431		7	max	545.526	2	3.636	6	5.276	5	0	1	0	12	-.002	15
432			min	-703.527	3	.844	15	-.472	1	0	4	-.019	4	-.009	4
433		8	max	545.356	2	2.767	6	5.881	5	0	1	0	12	-.003	15
434			min	-703.655	3	.64	15	-.472	1	0	4	-.016	4	-.01	4
435		9	max	545.185	2	1.898	6	6.486	5	0	1	0	12	-.003	15
436			min	-703.783	3	.435	15	-.472	1	0	4	-.014	4	-.012	4
437		10	max	545.015	2	1.029	6	7.091	5	0	1	0	12	-.003	15
438			min	-703.911	3	.231	15	-.472	1	0	4	-.01	4	-.012	4
439		11	max	544.845	2	.266	2	7.696	5	0	1	0	12	-.003	15
440			min	-704.038	3	-.059	3	-.472	1	0	4	-.007	4	-.012	4
441		12	max	544.674	2	-.177	15	8.301	5	0	1	0	12	-.003	15
442			min	-704.166	3	-.709	4	-.472	1	0	4	-.003	4	-.012	4
443		13	max	544.504	2	-.382	15	8.906	5	0	1	.002	5	-.003	15
444			min	-704.294	3	-1.578	4	-.472	1	0	4	-.003	1	-.012	4
445		14	max	544.334	2	-.586	15	9.511	5	0	1	.006	5	-.003	15
446			min	-704.422	3	-2.447	4	-.472	1	0	4	-.003	1	-.011	4
447		15	max	544.163	2	-.79	15	10.116	5	0	1	.011	5	-.002	15
448			min	-704.549	3	-3.316	4	-.472	1	0	4	-.004	1	-.01	4
449		16	max	543.993	2	-.994	15	10.722	5	0	1	.015	5	-.002	15
450			min	-704.677	3	-4.185	4	-.472	1	0	4	-.004	1	-.008	4
451		17	max	543.823	2	-1.199	15	11.327	5	0	1	.021	5	-.001	15
452			min	-704.805	3	-5.054	4	-.472	1	0	4	-.004	1	-.006	4
453		18	max	543.652	2	-1.403	15	11.932	5	0	1	.026	5	0	15
454			min	-704.933	3	-5.923	4	-.472	1	0	4	-.004	1	-.003	4
455		19	max	543.482	2	-1.607	15	12.537	5	0	1	.032	5	0	1
456			min	-705.06	3	-6.792	4	-.472	1	0	4	-.004	1	0	1
457	M12	1	max	1095.495	1	0	1	19.333	1	0	1	.026	5	0	1
458			min	10.918	12	0	1	-322.21	4	0	1	-.004	1	0	1
459		2	max	1095.665	1	0	1	19.333	1	0	1	0	12	0	1
460			min	11.003	12	0	1	-322.357	4	0	1	-.012	4	0	1
461		3	max	1095.835	1	0	1	19.333	1	0	1	0	1	0	1
462			min	11.088	12	0	1	-322.505	4	0	1	-.049	4	0	1
463		4	max	1096.006	1	0	1	19.333	1	0	1	.003	1	0	1
464			min	11.173	12	0	1	-322.653	4	0	1	-.086	4	0	1
465		5	max	1096.176	1	0	1	19.333	1	0	1	.005	1	0	1
466			min	11.259	12	0	1	-322.8	4	0	1	-.123	4	0	1
467		6	max	1096.346	1	0	1	19.333	1	0	1	.008	1	0	1
468			min	11.344	12	0	1	-322.948	4	0	1	-.16	4	0	1
469		7	max	1096.517	1	0	1	19.333	1	0	1	.01	1	0	1
470			min	11.429	12	0	1	-323.096	4	0	1	-.197	4	0	1
471		8	max	1096.687	1	0	1	19.333	1	0	1	.012	1	0	1
472			min	11.514	12	0	1	-323.243	4	0	1	-.234	4	0	1
473		9	max	1096.858	1	0	1	19.333	1	0	1	.014	1	0	1
474			min	11.599	12	0	1	-323.391	4	0	1	-.271	4	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1097.028	1	0	1	19.333	1	0	1	.016	1	0	1
476			min	11.684	12	0	1	-323.539	4	0	1	-.309	4	0	1
477		11	max	1097.198	1	0	1	19.333	1	0	1	.019	1	0	1
478			min	11.77	12	0	1	-323.686	4	0	1	-.346	4	0	1
479		12	max	1097.369	1	0	1	19.333	1	0	1	.021	1	0	1
480			min	11.855	12	0	1	-323.834	4	0	1	-.383	4	0	1
481		13	max	1097.539	1	0	1	19.333	1	0	1	.023	1	0	1
482			min	11.94	12	0	1	-323.981	4	0	1	-.42	4	0	1
483		14	max	1097.709	1	0	1	19.333	1	0	1	.025	1	0	1
484			min	12.025	12	0	1	-324.129	4	0	1	-.457	4	0	1
485		15	max	1097.88	1	0	1	19.333	1	0	1	.028	1	0	1
486			min	12.11	12	0	1	-324.277	4	0	1	-.495	4	0	1
487		16	max	1098.05	1	0	1	19.333	1	0	1	.03	1	0	1
488			min	12.196	12	0	1	-324.424	4	0	1	-.532	4	0	1
489		17	max	1098.22	1	0	1	19.333	1	0	1	.032	1	0	1
490			min	12.281	12	0	1	-324.572	4	0	1	-.569	4	0	1
491		18	max	1098.391	1	0	1	19.333	1	0	1	.034	1	0	1
492			min	12.366	12	0	1	-324.72	4	0	1	-.606	4	0	1
493		19	max	1098.561	1	0	1	19.333	1	0	1	.036	1	0	1
494			min	12.451	12	0	1	-324.867	4	0	1	-.644	4	0	1
495	M1	1	max	211.573	1	560.546	3	54.5	5	0	1	.304	1	0	3
496			min	-15.542	5	-407.43	1	-107.96	1	0	3	-.138	5	-.014	2
497		2	max	212.416	1	559.452	3	55.961	5	0	1	.237	1	.24	1
498			min	-15.149	5	-408.889	1	-107.96	1	0	3	-.104	5	-.348	3
499		3	max	450.049	3	474.072	1	21.5	5	0	3	.17	1	.485	1
500			min	-278.026	2	-413.089	3	-107.687	1	0	1	-.069	5	-.684	3
501		4	max	450.681	3	472.613	1	22.96	5	0	3	.103	1	.191	1
502			min	-277.183	2	-414.183	3	-107.687	1	0	1	-.055	5	-.427	3
503		5	max	451.313	3	471.154	1	24.42	5	0	3	.036	1	-.005	15
504			min	-276.341	2	-415.278	3	-107.687	1	0	1	-.041	5	-.17	3
505		6	max	451.945	3	469.695	1	25.88	5	0	3	-.002	12	.088	3
506			min	-275.499	2	-416.372	3	-107.687	1	0	1	-.032	4	-.399	2
507		7	max	452.576	3	468.236	1	27.341	5	0	3	-.006	12	.347	3
508			min	-274.656	2	-417.466	3	-107.687	1	0	1	-.097	1	-.685	1
509		8	max	453.208	3	466.777	1	28.801	5	0	3	.009	5	.606	3
510			min	-273.814	2	-418.561	3	-107.687	1	0	1	-.164	1	-.975	1
511		9	max	470.544	3	39.886	2	71.407	5	0	9	.101	1	.709	3
512			min	-182.065	2	.439	15	-166.393	1	0	3	-.172	5	-1.112	1
513		10	max	471.176	3	38.427	2	72.867	5	0	9	0	12	.691	3
514			min	-181.222	2	-.004	5	-166.393	1	0	3	-.129	4	-1.125	1
515		11	max	471.808	3	36.968	2	74.327	5	0	9	-.006	12	.674	3
516			min	-180.38	2	-1.791	4	-166.393	1	0	3	-.106	4	-1.138	1
517		12	max	489.037	3	274.645	3	186.806	5	0	2	.161	1	.588	3
518			min	-109.299	5	-526.187	2	-103.56	1	0	3	-.291	5	-1.007	2
519		13	max	489.668	3	273.551	3	188.266	5	0	2	.096	1	.418	3
520			min	-108.906	5	-527.646	2	-103.56	1	0	3	-.175	5	-.688	1
521		14	max	490.3	3	272.456	3	189.726	5	0	2	.032	1	.248	3
522			min	-108.513	5	-529.105	2	-103.56	1	0	3	-.058	5	-.37	1
523		15	max	490.932	3	271.362	3	191.186	5	0	2	.061	5	.08	3
524			min	-108.12	5	-530.564	2	-103.56	1	0	3	-.032	1	-.052	1
525		16	max	491.564	3	270.268	3	192.646	5	0	2	.18	5	.306	2
526			min	-107.726	5	-532.023	2	-103.56	1	0	3	-.096	1	-.088	3
527		17	max	492.196	3	269.173	3	194.106	5	0	2	.3	5	.637	2
528			min	-107.333	5	-533.482	2	-103.56	1	0	3	-.161	1	-.256	3
529		18	max	26.537	5	530.509	2	-6.817	12	0	5	.268	5	.319	2
530			min	-212.88	1	-218.954	3	-144.597	4	0	2	-.231	1	-.126	3
531		19	max	26.931	5	529.05	2	-6.817	12	0	5	.197	5	.011	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-212.038	1	-220.049	3	-143.137	4	0	2	-.307	1	-.011	1
533	M5	max	462.875	1	1866.72	3	111.683	5	0	1	0	1	.028	2
534		min	20.278	12	-1388.287	1	0	1	0	4	-.301	4	0	3
535		max	463.718	1	1865.626	3	113.143	5	0	1	0	1	.889	1
536		min	20.699	12	-1389.746	1	0	1	0	4	-.232	4	-1.158	3
537		max	1427.756	3	1385.797	1	83.441	4	0	4	0	1	1.721	1
538		min	-967.722	2	-1297.183	3	0	1	0	1	-.162	4	-2.281	3
539		max	1428.388	3	1384.338	1	84.901	4	0	4	0	1	.862	1
540		min	-966.88	2	-1298.277	3	0	1	0	1	-.109	4	-1.475	3
541		max	1429.02	3	1382.879	1	86.361	4	0	4	0	1	.024	9
542		min	-966.037	2	-1299.371	3	0	1	0	1	-.056	4	-.669	3
543		max	1429.652	3	1381.42	1	87.821	4	0	4	0	1	.138	3
544		min	-965.195	2	-1300.466	3	0	1	0	1	-.002	5	-.866	2
545		max	1430.283	3	1379.961	1	89.281	4	0	4	.053	4	.945	3
546		min	-964.352	2	-1301.56	3	0	1	0	1	0	1	-1.712	1
547		max	1430.915	3	1378.501	1	90.741	4	0	4	.109	4	1.753	3
548		min	-963.51	2	-1302.654	3	0	1	0	1	0	1	-2.568	1
549		max	1460.762	3	133.21	2	238.981	4	0	1	0	1	2.02	3
550		min	-773.985	2	.445	15	0	1	0	1	-.262	4	-2.914	1
551		max	1461.394	3	131.751	2	240.442	4	0	1	0	1	1.956	3
552		min	-773.143	2	.005	15	0	1	0	1	-.114	4	-2.958	1
553		max	1462.026	3	130.292	2	241.902	4	0	1	.036	4	1.892	3
554		min	-772.3	2	-1.458	6	0	1	0	1	0	1	-3.002	1
555		max	1492.086	3	843.347	3	265.718	4	0	1	0	1	1.66	3
556		min	-582.789	2	-1572.713	2	0	1	0	4	-.426	4	-2.682	2
557		max	1492.718	3	842.252	3	267.178	4	0	1	0	1	1.137	3
558		min	-581.946	2	-1574.172	2	0	1	0	4	-.26	4	-1.729	1
559		max	1493.35	3	841.158	3	268.638	4	0	1	0	1	.614	3
560		min	-581.104	2	-1575.631	2	0	1	0	4	-.094	4	-.782	1
561		max	1493.982	3	840.064	3	270.098	4	0	1	.073	4	.25	2
562		min	-580.261	2	-1577.09	2	0	1	0	4	0	1	0	13
563		max	1494.614	3	838.969	3	271.558	4	0	1	.241	4	1.229	2
564		min	-579.419	2	-1578.549	2	0	1	0	4	0	1	-.428	3
565		max	1495.245	3	837.875	3	273.018	4	0	1	.41	4	2.21	2
566		min	-578.577	2	-1580.008	2	0	1	0	4	0	1	-.949	3
567		max	-21.096	12	1794.164	2	0	1	0	4	.429	4	1.132	2
568		min	-462.789	1	-761.169	3	-28.098	5	0	1	0	1	-.494	3
569		max	-20.675	12	1792.705	2	0	1	0	4	.413	4	.022	1
570		min	-461.947	1	-762.263	3	-26.638	5	0	1	0	1	-.021	3
571	M9	max	211.573	1	560.546	3	107.96	1	0	3	-.018	12	0	3
572		min	10.735	12	-407.43	1	6.369	12	0	4	-.304	1	-.014	2
573		max	212.416	1	559.452	3	107.96	1	0	3	-.014	12	.24	1
574		min	11.157	12	-408.889	1	6.369	12	0	4	-.237	1	-.348	3
575		max	450.049	3	474.072	1	107.687	1	0	1	-.01	12	.485	1
576		min	-278.026	2	-413.089	3	6.336	12	0	3	-.17	1	-.684	3
577		max	450.681	3	472.613	1	107.687	1	0	1	-.006	12	.191	1
578		min	-277.183	2	-414.183	3	6.336	12	0	3	-.103	1	-.427	3
579		max	451.313	3	471.154	1	107.687	1	0	1	-.002	12	-.005	15
580		min	-276.341	2	-415.278	3	6.336	12	0	3	-.055	4	-.17	3
581		max	451.945	3	469.695	1	107.687	1	0	1	.031	1	.088	3
582		min	-275.499	2	-416.372	3	6.336	12	0	3	-.021	5	-.399	2
583		max	452.576	3	468.236	1	107.687	1	0	1	.097	1	.347	3
584		min	-274.656	2	-417.466	3	6.336	12	0	3	.003	15	-.685	1
585		max	453.208	3	466.777	1	107.687	1	0	1	.164	1	.606	3
586		min	-273.814	2	-418.561	3	6.336	12	0	3	.01	12	-.975	1
587		max	470.544	3	39.886	2	166.393	1	0	3	-.006	12	.709	3
588		min	-182.065	2	.454	15	9.541	12	0	9	-.212	4	-1.112	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	471.176	3	38.427	2	166.393	1	0	3	.002	1	.691	3
590		min	-181.222	2	.013	15	9.541	12	0	9	-.127	4	-1.125	1
591	11	max	471.808	3	36.968	2	166.393	1	0	3	.105	1	.674	3
592		min	-180.38	2	-1.676	6	9.541	12	0	9	-.066	5	-1.138	1
593	12	max	489.037	3	274.645	3	229.476	4	0	3	-.009	12	.588	3
594		min	-105.917	10	-526.187	2	5.779	12	0	2	-.357	4	-1.007	2
595	13	max	489.668	3	273.551	3	230.936	4	0	3	-.005	12	.418	3
596		min	-105.215	10	-527.646	2	5.779	12	0	2	-.214	4	-.688	1
597	14	max	490.3	3	272.456	3	232.396	4	0	3	-.002	12	.248	3
598		min	-104.513	10	-529.105	2	5.779	12	0	2	-.07	4	-.37	1
599	15	max	490.932	3	271.362	3	233.857	4	0	3	.074	4	.08	3
600		min	-103.811	10	-530.564	2	5.779	12	0	2	.002	12	-.052	1
601	16	max	491.564	3	270.268	3	235.317	4	0	3	.22	4	.306	2
602		min	-103.109	10	-532.023	2	5.779	12	0	2	.005	12	-.088	3
603	17	max	492.196	3	269.173	3	236.777	4	0	3	.367	4	.637	2
604		min	-102.407	10	-533.482	2	5.779	12	0	2	.009	12	-.256	3
605	18	max	-10.958	12	530.509	2	121.944	1	0	2	.36	4	.319	2
606		min	-212.88	1	-218.954	3	-97.887	5	0	3	.013	12	-.126	3
607	19	max	-10.537	12	529.05	2	121.944	1	0	2	.317	4	.011	3
608		min	-212.038	1	-220.049	3	-96.427	5	0	3	.017	12	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	.001	1	.162	2	.009	3	1.117e-2	2	NC	1	NC	1
2				min	-1.035	4	-.03	3	-.004	2	-2.09e-3	3	NC	1	NC
3		2	max	0	1	.236	3	.052	1	1.253e-2	2	NC	5	NC	2
4			min	-1.035	4	-.01	9	-.031	5	-2.067e-3	3	902.356	3	4777.629	1
5		3	max	0	1	.452	3	.122	1	1.388e-2	2	NC	5	NC	3
6			min	-1.035	4	-.138	1	-.037	5	-2.045e-3	3	498.484	3	1995.089	1
7		4	max	0	1	.583	3	.182	1	1.524e-2	2	NC	5	NC	3
8			min	-1.035	4	-.208	1	-.027	5	-2.022e-3	3	391.805	3	1332.898	1
9		5	max	0	1	.614	3	.212	1	1.66e-2	2	NC	5	NC	3
10			min	-1.035	4	-.206	1	-.007	5	-1.999e-3	3	373.023	3	1141.955	1
11		6	max	0	1	.547	3	.204	1	1.795e-2	2	NC	5	NC	5
12			min	-1.035	4	-.134	1	.009	15	-1.976e-3	3	416.515	3	1187.976	1
13		7	max	0	1	.402	3	.159	1	1.931e-2	2	NC	5	NC	10
14			min	-1.035	4	-.016	9	.011	10	-1.954e-3	3	556.464	3	1520.835	1
15		8	max	0	1	.217	3	.092	1	2.066e-2	2	NC	1	NC	3
16			min	-1.035	4	.005	15	.002	10	-1.931e-3	3	971.624	3	2653.183	1
17		9	max	0	1	.286	2	.038	4	2.202e-2	2	NC	4	NC	1
18			min	-1.035	4	.009	15	-.008	10	-1.908e-3	3	1936.066	2	6252.889	4
19		10	max	0	1	.34	2	.027	3	2.337e-2	2	NC	3	NC	1
20			min	-1.035	4	-.026	3	-.019	2	-1.885e-3	3	1347.314	2	NC	1
21		11	max	0	12	.286	2	.029	3	2.202e-2	2	NC	4	NC	1
22			min	-1.035	4	.009	15	-.024	5	-1.908e-3	3	1936.066	2	NC	1
23		12	max	0	12	.217	3	.092	1	2.066e-2	2	NC	1	NC	3
24			min	-1.035	4	.005	15	-.023	5	-1.931e-3	3	971.624	3	2653.183	1
25		13	max	0	12	.402	3	.159	1	1.931e-2	2	NC	5	NC	5
26			min	-1.035	4	-.016	9	-.007	5	-1.954e-3	3	556.464	3	1520.835	1
27		14	max	0	12	.547	3	.204	1	1.795e-2	2	NC	5	NC	5
28			min	-1.035	4	-.134	1	.01	15	-1.976e-3	3	416.515	3	1187.976	1
29		15	max	0	12	.614	3	.212	1	1.66e-2	2	NC	5	NC	3
30			min	-1.035	4	-.206	1	.02	10	-1.999e-3	3	373.023	3	1141.955	1
31		16	max	0	12	.583	3	.182	1	1.524e-2	2	NC	5	NC	3
32			min	-1.035	4	-.208	1	.017	12	-2.022e-3	3	391.805	3	1332.898	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.452	3	.122	1	1.388e-2	2	NC	5	NC	3
34		min	-1.035	4	-1.138	1	.011	10	-2.045e-3	3	498.484	3	1995.089	1
35	18	max	0	12	.236	3	.052	4	1.253e-2	2	NC	5	NC	2
36		min	-1.035	4	-.01	9	.003	10	-2.067e-3	3	902.356	3	4527.551	4
37	19	max	0	12	.162	2	.009	3	1.117e-2	2	NC	1	NC	1
38		min	-1.035	4	-.03	3	-.004	2	-2.09e-3	3	NC	1	NC	1
39	M14	1	max	0	.292	3	.008	3	6.588e-3	1	NC	1	NC	1
40		min	-.748	4	-.506	1	-.004	2	-4.437e-3	3	NC	1	NC	1
41	2	max	0	1	.579	3	.035	1	7.776e-3	1	NC	5	NC	2
42		min	-.748	4	-.832	1	-.046	5	-5.338e-3	3	734.796	1	5462.987	5
43	3	max	0	1	.825	3	.095	1	8.964e-3	1	NC	15	NC	3
44		min	-.748	4	-1.118	1	-.055	5	-6.239e-3	3	391.806	1	2562.548	1
45	4	max	0	1	1.003	3	.153	1	1.015e-2	1	9324.955	15	NC	3
46		min	-.748	4	-1.335	1	-.038	5	-7.14e-3	3	289.369	1	1591.224	1
47	5	max	0	1	1.099	3	.185	1	1.134e-2	1	8068.611	15	NC	3
48		min	-.748	4	-1.468	1	-.007	5	-8.04e-3	3	249.434	1	1309.435	1
49	6	max	0	1	1.111	3	.183	1	1.253e-2	1	7732.605	15	NC	3
50		min	-.748	4	-1.515	1	.016	10	-8.941e-3	3	237.774	1	1327.561	1
51	7	max	0	1	1.054	3	.146	1	1.372e-2	1	7999.255	15	NC	3
52		min	-.748	4	-1.488	1	.01	10	-9.842e-3	3	244.196	1	1668.536	1
53	8	max	0	1	.954	3	.088	4	1.49e-2	1	8740.244	15	NC	3
54		min	-.748	4	-1.414	1	.002	10	-1.074e-2	3	264.362	1	2703.512	4
55	9	max	0	1	.853	3	.058	4	1.609e-2	1	9734.342	15	NC	1
56		min	-.748	4	-1.329	1	-.007	10	-1.164e-2	3	291.559	1	4068.488	4
57	10	max	0	1	.804	3	.024	3	1.728e-2	1	NC	15	NC	1
58		min	-.748	4	-1.287	1	-.017	2	-1.254e-2	3	307.336	1	NC	1
59	11	max	0	12	.853	3	.025	3	1.609e-2	1	9734.306	15	NC	1
60		min	-.748	4	-1.329	1	-.045	5	-1.164e-2	3	291.559	1	5580.726	5
61	12	max	0	12	.954	3	.085	1	1.49e-2	1	8740.14	15	NC	3
62		min	-.748	4	-1.414	1	-.051	5	-1.074e-2	3	264.362	1	2866.081	1
63	13	max	0	12	1.054	3	.146	1	1.372e-2	1	7999.085	15	NC	3
64		min	-.748	4	-1.488	1	-.033	5	-9.842e-3	3	244.196	1	1668.536	1
65	14	max	0	12	1.111	3	.183	1	1.253e-2	1	7732.367	15	NC	3
66		min	-.748	4	-1.515	1	0	15	-8.941e-3	3	237.774	1	1327.561	1
67	15	max	0	12	1.099	3	.185	1	1.134e-2	1	8068.287	15	NC	3
68		min	-.748	4	-1.468	1	.018	10	-8.04e-3	3	249.434	1	1309.435	1
69	16	max	0	12	1.003	3	.153	1	1.015e-2	1	9324.488	15	NC	3
70		min	-.748	4	-1.335	1	.014	10	-7.14e-3	3	289.369	1	1591.224	1
71	17	max	0	12	.825	3	.095	1	8.964e-3	1	NC	15	NC	3
72		min	-.748	4	-1.118	1	.008	10	-6.239e-3	3	391.806	1	2551.469	4
73	18	max	0	12	.579	3	.061	4	7.776e-3	1	NC	5	NC	2
74		min	-.748	4	-.832	1	0	10	-5.338e-3	3	734.796	1	3898.992	4
75	19	max	0	12	.292	3	.008	3	6.588e-3	1	NC	1	NC	1
76		min	-.748	4	-.506	1	-.004	2	-4.437e-3	3	NC	1	NC	1
77	M15	1	max	0	.299	3	.007	3	3.767e-3	3	NC	1	NC	1
78		min	-.592	4	-.505	1	-.004	2	-6.758e-3	1	NC	1	NC	1
79	2	max	0	12	.492	3	.035	1	4.533e-3	3	NC	5	NC	2
80		min	-.592	4	-.88	2	-.061	5	-7.987e-3	1	631.745	2	4055.39	5
81	3	max	0	12	.663	3	.096	1	5.299e-3	3	NC	15	NC	3
82		min	-.592	4	-1.209	2	-.074	5	-9.216e-3	1	338.697	2	2549.828	1
83	4	max	0	12	.796	3	.153	1	6.066e-3	3	9340.673	15	NC	3
84		min	-.592	4	-1.451	2	-.054	5	-1.044e-2	1	252.457	2	1585.214	1
85	5	max	0	12	.882	3	.186	1	6.832e-3	3	8083.857	15	NC	3
86		min	-.592	4	-1.588	2	-.014	5	-1.167e-2	1	220.618	2	1305.068	1
87	6	max	0	12	.92	3	.183	1	7.598e-3	3	7749.439	15	NC	3
88		min	-.592	4	-1.62	2	.017	10	-1.29e-2	1	214.445	2	1323.104	1
89	7	max	0	12	.916	3	.146	1	8.365e-3	3	8019.811	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-592	4	-1.561	2	.011	10	-1.413e-2	1	226.25	2	1661.885	1
91	8	max	0	12	.883	3	.104	4	9.131e-3	3	8767.111	15	NC	3
92		min	-592	4	-1.446	2	.002	10	-1.536e-2	1	253.682	2	2297.445	4
93	9	max	0	12	.843	3	.071	4	9.897e-3	3	9769.52	15	NC	1
94		min	-592	4	-1.337	1	-.006	10	-1.659e-2	1	288.493	1	3335.125	4
95	10	max	0	1	.822	3	.022	3	1.066e-2	3	NC	15	NC	1
96		min	-592	4	-1.284	1	-.016	2	-1.782e-2	1	308.065	1	NC	1
97	11	max	0	1	.843	3	.025	1	9.897e-3	3	9769.489	15	NC	1
98		min	-592	4	-1.337	1	-.058	5	-1.659e-2	1	288.493	1	4278.464	5
99	12	max	0	1	.883	3	.086	1	9.131e-3	3	8767.032	15	NC	3
100		min	-592	4	-1.446	2	-.067	5	-1.536e-2	1	253.682	2	2848.035	1
101	13	max	0	1	.916	3	.146	1	8.365e-3	3	8019.689	15	NC	3
102		min	-592	4	-1.561	2	-.044	5	-1.413e-2	1	226.25	2	1661.885	1
103	14	max	0	1	.92	3	.183	1	7.598e-3	3	7749.272	15	NC	3
104		min	-592	4	-1.62	2	-.003	5	-1.29e-2	1	214.445	2	1323.104	1
105	15	max	0	1	.882	3	.186	1	6.832e-3	3	8083.632	15	NC	3
106		min	-592	4	-1.588	2	.017	12	-1.167e-2	1	220.618	2	1305.068	1
107	16	max	0	1	.796	3	.153	1	6.066e-3	3	9340.351	15	NC	3
108		min	-592	4	-1.451	2	.014	12	-1.044e-2	1	252.457	2	1585.214	1
109	17	max	0	1	.663	3	.113	4	5.299e-3	3	NC	15	NC	3
110		min	-592	4	-1.209	2	.008	10	-9.216e-3	1	338.697	2	2110.068	4
111	18	max	0	1	.492	3	.076	4	4.533e-3	3	NC	5	NC	2
112		min	-592	4	-.88	2	.001	10	-7.987e-3	1	631.745	2	3123.155	4
113	19	max	0	1	.299	3	.007	3	3.767e-3	3	NC	1	NC	1
114		min	-592	4	-.505	1	-.004	2	-6.758e-3	1	NC	1	NC	1
115	M16	max	0	12	.156	1	.006	3	6.816e-3	3	NC	1	NC	1
116		min	-149	4	-.101	3	-.003	2	-1.007e-2	1	NC	1	NC	1
117	2	max	0	12	.003	13	.051	1	7.858e-3	3	NC	5	NC	2
118		min	-149	4	-.084	2	-.044	5	-1.116e-2	1	1048.675	2	4843.17	1
119	3	max	0	12	.045	3	.121	1	8.9e-3	3	NC	5	NC	3
120		min	-149	4	-.265	2	-.055	5	-1.226e-2	1	584.996	2	2009.928	1
121	4	max	0	12	.075	3	.181	1	9.942e-3	3	NC	5	NC	3
122		min	-149	4	-.367	2	-.042	5	-1.336e-2	1	468.286	2	1338.32	1
123	5	max	0	12	.067	3	.211	1	1.098e-2	3	NC	5	NC	3
124		min	-149	4	-.376	2	-.015	5	-1.445e-2	1	460.547	2	1143.633	1
125	6	max	0	12	.022	3	.204	1	1.203e-2	3	NC	5	NC	3
126		min	-149	4	-.294	2	.01	15	-1.555e-2	1	546.929	2	1186.386	1
127	7	max	0	12	.002	13	.16	1	1.307e-2	3	NC	5	NC	3
128		min	-149	4	-.14	2	.013	10	-1.664e-2	1	840.579	2	1512.179	1
129	8	max	0	12	.093	1	.093	1	1.411e-2	3	NC	4	NC	3
130		min	-149	4	-.137	3	.004	10	-1.774e-2	1	2440.025	2	2610.33	1
131	9	max	0	12	.249	1	.048	4	1.515e-2	3	NC	4	NC	2
132		min	-149	4	-.212	3	-.005	10	-1.883e-2	1	2170.165	3	4951.718	4
133	10	max	0	1	.319	1	.019	3	1.619e-2	3	NC	5	NC	1
134		min	-149	4	-.245	3	-.014	2	-1.993e-2	1	1472.43	1	NC	1
135	11	max	0	1	.249	1	.027	1	1.515e-2	3	NC	4	NC	2
136		min	-149	4	-.212	3	-.034	5	-1.883e-2	1	2170.165	3	7327.546	5
137	12	max	0	1	.093	1	.093	1	1.411e-2	3	NC	4	NC	3
138		min	-148	4	-.137	3	-.035	5	-1.774e-2	1	2440.025	2	2610.33	1
139	13	max	0	1	.002	13	.16	1	1.307e-2	3	NC	5	NC	3
140		min	-148	4	-.14	2	-.015	5	-1.664e-2	1	840.579	2	1512.179	1
141	14	max	0	1	.022	3	.204	1	1.203e-2	3	NC	5	NC	3
142		min	-148	4	-.294	2	.01	15	-1.555e-2	1	546.929	2	1186.386	1
143	15	max	0	1	.067	3	.211	1	1.098e-2	3	NC	5	NC	3
144		min	-148	4	-.376	2	.017	12	-1.445e-2	1	460.547	2	1143.633	1
145	16	max	0	1	.075	3	.181	1	9.942e-3	3	NC	5	NC	3
146		min	-148	4	-.367	2	.015	12	-1.336e-2	1	468.286	2	1338.32	1



Company : Schletter, Inc.  
Designer : HCV  
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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.045	3	.121	1	8.9e-3	3	NC	5	NC	3
148			min	-.148	4	-.265	2	.011	12	-1.226e-2	1	584.996	2	2009.928	1
149		18	max	.001	1	.002	13	.065	4	7.858e-3	3	NC	5	NC	2
150			min	-.148	4	-.084	2	.003	10	-1.116e-2	1	1048.675	2	3644.517	4
151		19	max	.001	1	.156	1	.006	3	6.816e-3	3	NC	1	NC	1
152			min	-.148	4	-.101	3	-.003	2	-1.007e-2	1	NC	1	NC	1
153	M2	1	max	.007	1	.008	2	.014	1	2.699e-3	5	NC	1	NC	2
154			min	-.008	3	-.013	3	-.964	4	-3.333e-4	1	9894.859	2	80.364	4
155		2	max	.007	1	.006	2	.013	1	2.762e-3	5	NC	1	NC	2
156			min	-.007	3	-.013	3	-.886	4	-3.164e-4	1	NC	1	87.484	4
157		3	max	.006	1	.005	2	.012	1	2.824e-3	5	NC	1	NC	2
158			min	-.007	3	-.013	3	-.808	4	-2.995e-4	1	NC	1	95.936	4
159		4	max	.006	1	.004	2	.011	1	2.886e-3	5	NC	1	NC	2
160			min	-.007	3	-.012	3	-.731	4	-2.825e-4	1	NC	1	106.065	4
161		5	max	.006	1	.003	2	.01	1	2.948e-3	5	NC	1	NC	2
162			min	-.006	3	-.012	3	-.655	4	-2.656e-4	1	NC	1	118.345	4
163		6	max	.005	1	.001	2	.008	1	3.01e-3	5	NC	1	NC	2
164			min	-.006	3	-.012	3	-.581	4	-2.487e-4	1	NC	1	133.425	4
165		7	max	.005	1	0	2	.007	1	3.072e-3	5	NC	1	NC	1
166			min	-.005	3	-.011	3	-.509	4	-2.317e-4	1	NC	1	152.232	4
167		8	max	.004	1	0	15	.006	1	3.134e-3	5	NC	1	NC	1
168			min	-.005	3	-.011	3	-.44	4	-2.148e-4	1	NC	1	176.113	4
169		9	max	.004	1	0	15	.005	1	3.198e-3	4	NC	1	NC	1
170			min	-.004	3	-.01	3	-.374	4	-1.979e-4	1	NC	1	207.094	4
171		10	max	.004	1	-.001	15	.005	1	3.266e-3	4	NC	1	NC	1
172			min	-.004	3	-.01	3	-.312	4	-1.81e-4	1	NC	1	248.338	4
173		11	max	.003	1	-.001	15	.004	1	3.334e-3	4	NC	1	NC	1
174			min	-.004	3	-.009	3	-.254	4	-1.64e-4	1	NC	1	305.014	4
175		12	max	.003	1	-.001	15	.003	1	3.402e-3	4	NC	1	NC	1
176			min	-.003	3	-.008	3	-.201	4	-1.471e-4	1	NC	1	386.048	4
177		13	max	.002	1	-.001	15	.002	1	3.47e-3	4	NC	1	NC	1
178			min	-.003	3	-.007	3	-.153	4	-1.302e-4	1	NC	1	507.975	4
179		14	max	.002	1	-.001	15	.002	1	3.538e-3	4	NC	1	NC	1
180			min	-.002	3	-.006	3	-.11	4	-1.132e-4	1	NC	1	704.35	4
181		15	max	.002	1	-.001	15	.001	1	3.606e-3	4	NC	1	NC	1
182			min	-.002	3	-.005	3	-.074	4	-9.632e-5	1	NC	1	1052.271	4
183		16	max	.001	1	0	15	0	1	3.674e-3	4	NC	1	NC	1
184			min	-.001	3	-.004	6	-.044	4	-7.939e-5	1	NC	1	1763.875	4
185		17	max	0	1	0	15	0	1	3.742e-3	4	NC	1	NC	1
186			min	0	3	-.003	6	-.021	4	-6.246e-5	1	NC	1	3620.01	4
187		18	max	0	1	0	15	0	1	3.81e-3	4	NC	1	NC	1
188			min	0	3	-.002	6	-.007	4	-4.553e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	3.878e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	-2.86e-5	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	5.421e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	-8.42e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.021	4	4.18e-5	1	NC	1	NC	1
194			min	0	2	-.003	6	0	1	-2.535e-5	5	NC	1	NC	1
195		3	max	0	3	-.001	15	.04	4	8.114e-4	4	NC	1	NC	1
196			min	0	2	-.006	6	0	1	4.315e-6	12	NC	1	7954.73	5
197		4	max	.001	3	-.002	15	.058	4	1.638e-3	4	NC	1	NC	1
198			min	0	2	-.009	6	0	1	6.297e-6	12	NC	1	6002.957	5
199		5	max	.002	3	-.003	15	.074	4	2.465e-3	4	NC	1	NC	1
200			min	-.001	2	-.012	6	0	1	8.279e-6	12	8520.357	6	5157.93	5
201		6	max	.002	3	-.003	15	.089	4	3.292e-3	4	NC	5	NC	1
202			min	-.001	2	-.015	6	0	1	1.026e-5	12	6904.454	6	4794.105	5
203		7	max	.002	3	-.004	15	.103	4	4.118e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.017	6	0	1	1.224e-5	12	5931.236	6	4719.521	5
205		8	max	.003	3	-.004	15	.116	4	4.945e-3	4	NC	5	NC	1
206			min	-.002	2	-.019	6	0	12	1.423e-5	12	5331.006	6	4876.016	5
207		9	max	.003	3	-.005	15	.128	4	5.772e-3	4	NC	5	NC	1
208			min	-.002	2	-.021	6	0	12	1.621e-5	12	4976.818	6	5272.409	5
209		10	max	.003	3	-.005	15	.139	4	6.599e-3	4	NC	5	NC	1
210			min	-.003	2	-.021	6	0	12	1.819e-5	12	4807.129	6	5978.494	5
211		11	max	.004	3	-.005	15	.15	4	7.425e-3	4	NC	5	NC	1
212			min	-.003	2	-.021	6	0	12	2.017e-5	12	4796.991	6	7159.244	5
213		12	max	.004	3	-.005	15	.161	4	8.252e-3	4	NC	5	NC	1
214			min	-.003	2	-.021	6	0	12	2.215e-5	12	4948.492	6	9186.08	5
215		13	max	.005	3	-.004	15	.171	4	9.079e-3	4	NC	5	NC	1
216			min	-.004	2	-.019	6	0	12	2.414e-5	12	5292.785	6	NC	1
217		14	max	.005	3	-.004	15	.182	4	9.905e-3	4	NC	5	NC	1
218			min	-.004	2	-.017	6	0	12	2.612e-5	12	5906.173	6	NC	1
219		15	max	.005	3	-.003	15	.193	4	1.073e-2	4	NC	3	NC	1
220			min	-.004	2	-.015	6	0	12	2.81e-5	12	6957.519	6	NC	1
221		16	max	.006	3	-.002	15	.204	4	1.156e-2	4	NC	1	NC	1
222			min	-.004	2	-.012	6	0	12	3.008e-5	12	8855.603	6	NC	1
223		17	max	.006	3	-.002	15	.217	4	1.239e-2	4	NC	1	NC	1
224			min	-.005	2	-.008	1	0	12	3.206e-5	12	NC	1	NC	1
225		18	max	.007	3	0	15	.231	4	1.321e-2	4	NC	1	NC	2
226			min	-.005	2	-.005	1	0	12	3.405e-5	12	NC	1	9279.418	1
227		19	max	.007	3	0	5	.247	4	1.404e-2	4	NC	1	NC	2
228			min	-.005	2	-.002	1	0	12	3.603e-5	12	NC	1	7741.692	1
229	M4	1	max	.003	1	.005	2	0	12	2.338e-4	1	NC	1	NC	3
230			min	0	5	-.007	3	-.247	4	-6.414e-4	5	NC	1	100.619	4
231		2	max	.002	1	.005	2	0	12	2.338e-4	1	NC	1	NC	3
232			min	0	5	-.007	3	-.227	4	-6.414e-4	5	NC	1	109.447	4
233		3	max	.002	1	.004	2	0	12	2.338e-4	1	NC	1	NC	3
234			min	0	5	-.006	3	-.207	4	-6.414e-4	5	NC	1	119.95	4
235		4	max	.002	1	.004	2	0	12	2.338e-4	1	NC	1	NC	3
236			min	0	5	-.006	3	-.187	4	-6.414e-4	5	NC	1	132.565	4
237		5	max	.002	1	.004	2	0	12	2.338e-4	1	NC	1	NC	3
238			min	0	5	-.006	3	-.168	4	-6.414e-4	5	NC	1	147.885	4
239		6	max	.002	1	.004	2	0	12	2.338e-4	1	NC	1	NC	3
240			min	0	5	-.005	3	-.149	4	-6.414e-4	5	NC	1	166.729	4
241		7	max	.002	1	.003	2	0	12	2.338e-4	1	NC	1	NC	3
242			min	0	5	-.005	3	-.13	4	-6.414e-4	5	NC	1	190.265	4
243		8	max	.002	1	.003	2	0	12	2.338e-4	1	NC	1	NC	3
244			min	0	5	-.004	3	-.113	4	-6.414e-4	5	NC	1	220.191	4
245		9	max	.001	1	.003	2	0	12	2.338e-4	1	NC	1	NC	2
246			min	0	5	-.004	3	-.096	4	-6.414e-4	5	NC	1	259.067	4
247		10	max	.001	1	.003	2	0	12	2.338e-4	1	NC	1	NC	2
248			min	0	5	-.004	3	-.08	4	-6.414e-4	5	NC	1	310.894	4
249		11	max	.001	1	.002	2	0	12	2.338e-4	1	NC	1	NC	2
250			min	0	5	-.003	3	-.065	4	-6.414e-4	5	NC	1	382.217	4
251		12	max	.001	1	.002	2	0	12	2.338e-4	1	NC	1	NC	2
252			min	0	5	-.003	3	-.051	4	-6.414e-4	5	NC	1	484.357	4
253		13	max	0	1	.002	2	0	12	2.338e-4	1	NC	1	NC	1
254			min	0	5	-.002	3	-.039	4	-6.414e-4	5	NC	1	638.328	4
255		14	max	0	1	.001	2	0	12	2.338e-4	1	NC	1	NC	1
256			min	0	5	-.002	3	-.028	4	-6.414e-4	5	NC	1	886.881	4
257		15	max	0	1	.001	2	0	12	2.338e-4	1	NC	1	NC	1
258			min	0	5	-.002	3	-.019	4	-6.414e-4	5	NC	1	1328.564	4
259		16	max	0	1	0	2	0	12	2.338e-4	1	NC	1	NC	1
260			min	0	5	-.001	3	-.011	4	-6.414e-4	5	NC	1	2235.791	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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Nov 4, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	2.338e-4	1	NC	1	NC	1
262			min	0	5	0	3	-0.005	4	-6.414e-4	5	NC	1	4618.582	4
263		18	max	0	1	0	2	0	12	2.338e-4	1	NC	1	NC	1
264			min	0	5	0	3	-0.002	4	-6.414e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.338e-4	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-6.414e-4	5	NC	1	NC	1
267	M6	1	max	.023	1	.03	2	0	1	2.864e-3	4	NC	3	NC	1
268			min	-.026	3	-.041	3	-.975	4	0	1	2584.591	2	79.483	4
269		2	max	.022	1	.027	2	0	1	2.923e-3	4	NC	3	NC	1
270			min	-.024	3	-.039	3	-.896	4	0	1	2858.291	2	86.525	4
271		3	max	.02	1	.024	2	0	1	2.983e-3	4	NC	3	NC	1
272			min	-.023	3	-.037	3	-.817	4	0	1	3193.536	2	94.885	4
273		4	max	.019	1	.021	2	0	1	3.042e-3	4	NC	3	NC	1
274			min	-.021	3	-.035	3	-.739	4	0	1	3609.563	2	104.904	4
275		5	max	.018	1	.019	2	0	1	3.102e-3	4	NC	3	NC	1
276			min	-.02	3	-.032	3	-.662	4	0	1	4134.052	2	117.049	4
277		6	max	.017	1	.016	2	0	1	3.161e-3	4	NC	3	NC	1
278			min	-.018	3	-.03	3	-.587	4	0	1	4808.046	2	131.966	4
279		7	max	.015	1	.014	2	0	1	3.221e-3	4	NC	1	NC	1
280			min	-.017	3	-.028	3	-.515	4	0	1	5694.61	2	150.568	4
281		8	max	.014	1	.011	2	0	1	3.28e-3	4	NC	1	NC	1
282			min	-.016	3	-.026	3	-.445	4	0	1	6894.937	2	174.188	4
283		9	max	.013	1	.009	2	0	1	3.34e-3	4	NC	1	NC	1
284			min	-.014	3	-.024	3	-.378	4	0	1	8580.393	2	204.83	4
285		10	max	.011	1	.007	2	0	1	3.399e-3	4	NC	1	NC	1
286			min	-.013	3	-.022	3	-.315	4	0	1	NC	1	245.624	4
287		11	max	.01	1	.005	2	0	1	3.459e-3	4	NC	1	NC	1
288			min	-.011	3	-.019	3	-.257	4	0	1	NC	1	301.68	4
289		12	max	.009	1	.004	2	0	1	3.518e-3	4	NC	1	NC	1
290			min	-.01	3	-.017	3	-.203	4	0	1	NC	1	381.826	4
291		13	max	.008	1	.002	2	0	1	3.578e-3	4	NC	1	NC	1
292			min	-.009	3	-.015	3	-.154	4	0	1	NC	1	502.412	4
293		14	max	.006	1	.001	2	0	1	3.637e-3	4	NC	1	NC	1
294			min	-.007	3	-.012	3	-.111	4	0	1	NC	1	696.619	4
295		15	max	.005	1	0	2	0	1	3.697e-3	4	NC	1	NC	1
296			min	-.006	3	-.01	3	-.074	4	0	1	NC	1	1040.675	4
297		16	max	.004	1	0	2	0	1	3.756e-3	4	NC	1	NC	1
298			min	-.004	3	-.007	3	-.044	4	0	1	NC	1	1744.301	4
299		17	max	.003	1	0	2	0	1	3.816e-3	4	NC	1	NC	1
300			min	-.003	3	-.005	3	-.022	4	0	1	NC	1	3579.28	4
301		18	max	.001	1	0	2	0	1	3.875e-3	4	NC	1	NC	1
302			min	-.001	3	-.003	3	-.007	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	3.935e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-8.549e-4	4	NC	1	NC	1
307		2	max	.001	3	0	15	.021	4	0	1	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	-5.615e-5	4	NC	1	NC	1
309		3	max	.002	3	-.001	15	.041	4	7.426e-4	4	NC	1	NC	1
310			min	-.002	2	-.007	3	0	1	0	1	NC	1	7071.216	4
311		4	max	.004	3	-.002	15	.059	4	1.541e-3	4	NC	1	NC	1
312			min	-.003	2	-.01	3	0	1	0	1	NC	1	5282.925	4
313		5	max	.005	3	-.003	15	.075	4	2.34e-3	4	NC	1	NC	1
314			min	-.005	2	-.012	3	0	1	0	1	8575.985	4	4486.358	4
315		6	max	.006	3	-.004	15	.09	4	3.139e-3	4	NC	1	NC	1
316			min	-.006	2	-.015	4	0	1	0	1	6945.504	4	4112.418	4
317		7	max	.007	3	-.004	15	.104	4	3.937e-3	4	NC	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.007	2	-.018	4	0	1	0	1	5963.694	4	3981.329	4
319	8	max	.008	3	-.005	15	.117	4	4.736e-3	4	NC	5	NC	1
320		min	-.008	2	-.02	4	0	1	0	1	5358.103	4	4029.738	4
321	9	max	.009	3	-.005	15	.129	4	5.535e-3	4	NC	5	NC	1
322		min	-.009	2	-.021	4	0	1	0	1	5000.5	4	4246.099	4
323	10	max	.011	3	-.005	15	.14	4	6.334e-3	4	NC	5	NC	1
324		min	-.01	2	-.022	4	0	1	0	1	4828.695	4	4655.653	4
325	11	max	.012	3	-.005	15	.15	4	7.132e-3	4	NC	5	NC	1
326		min	-.011	2	-.022	4	0	1	0	1	4817.411	4	5327.218	4
327	12	max	.013	3	-.005	15	.16	4	7.931e-3	4	NC	5	NC	1
328		min	-.013	2	-.021	4	0	1	0	1	4968.599	4	6404.114	4
329	13	max	.014	3	-.005	15	.17	4	8.73e-3	4	NC	5	NC	1
330		min	-.014	2	-.02	4	0	1	0	1	5313.43	4	8188.28	4
331	14	max	.015	3	-.004	15	.179	4	9.528e-3	4	NC	5	NC	1
332		min	-.015	2	-.018	4	0	1	0	1	5928.411	4	NC	1
333	15	max	.017	3	-.004	15	.189	4	1.033e-2	4	NC	1	NC	1
334		min	-.016	2	-.016	4	0	1	0	1	6982.947	4	NC	1
335	16	max	.018	3	-.003	15	.199	4	1.113e-2	4	NC	1	NC	1
336		min	-.017	2	-.013	3	0	1	0	1	8887.199	4	NC	1
337	17	max	.019	3	-.002	15	.21	4	1.192e-2	4	NC	1	NC	1
338		min	-.018	2	-.011	3	0	1	0	1	NC	1	NC	1
339	18	max	.02	3	-.001	15	.222	4	1.272e-2	4	NC	1	NC	1
340		min	-.019	2	-.008	3	0	1	0	1	NC	1	NC	1
341	19	max	.021	3	0	10	.236	4	1.352e-2	4	NC	1	NC	1
342		min	-.02	2	-.005	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.007	1	.02	2	0	0	1	NC	1	NC	1
344		min	0	3	-.022	3	-.236	4	-8.699e-4	4	NC	1	105.252	4
345	2	max	.006	1	.019	2	0	1	0	1	NC	1	NC	1
346		min	0	3	-.021	3	-.217	4	-8.699e-4	4	NC	1	114.505	4
347	3	max	.006	1	.018	2	0	1	0	1	NC	1	NC	1
348		min	0	3	-.019	3	-.198	4	-8.699e-4	4	NC	1	125.514	4
349	4	max	.006	1	.016	2	0	1	0	1	NC	1	NC	1
350		min	0	3	-.018	3	-.179	4	-8.699e-4	4	NC	1	138.736	4
351	5	max	.005	1	.015	2	0	1	0	1	NC	1	NC	1
352		min	0	3	-.017	3	-.16	4	-8.699e-4	4	NC	1	154.79	4
353	6	max	.005	1	.014	2	0	1	0	1	NC	1	NC	1
354		min	0	3	-.016	3	-.142	4	-8.699e-4	4	NC	1	174.538	4
355	7	max	.004	1	.013	2	0	1	0	1	NC	1	NC	1
356		min	0	3	-.015	3	-.125	4	-8.699e-4	4	NC	1	199.201	4
357	8	max	.004	1	.012	2	0	1	0	1	NC	1	NC	1
358		min	0	3	-.013	3	-.108	4	-8.699e-4	4	NC	1	230.561	4
359	9	max	.004	1	.011	2	0	1	0	1	NC	1	NC	1
360		min	0	3	-.012	3	-.091	4	-8.699e-4	4	NC	1	271.3	4
361	10	max	.003	1	.01	2	0	1	0	1	NC	1	NC	1
362		min	0	3	-.011	3	-.076	4	-8.699e-4	4	NC	1	325.611	4
363	11	max	.003	1	.009	2	0	1	0	1	NC	1	NC	1
364		min	0	3	-.01	3	-.062	4	-8.699e-4	4	NC	1	400.354	4
365	12	max	.003	1	.008	2	0	1	0	1	NC	1	NC	1
366		min	0	3	-.009	3	-.049	4	-8.699e-4	4	NC	1	507.394	4
367	13	max	.002	1	.007	2	0	1	0	1	NC	1	NC	1
368		min	0	3	-.007	3	-.037	4	-8.699e-4	4	NC	1	668.757	4
369	14	max	.002	1	.005	2	0	1	0	1	NC	1	NC	1
370		min	0	3	-.006	3	-.027	4	-8.699e-4	4	NC	1	929.253	4
371	15	max	.001	1	.004	2	0	1	0	1	NC	1	NC	1
372		min	0	3	-.005	3	-.018	4	-8.699e-4	4	NC	1	1392.179	4
373	16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374		min	0	3	-.004	3	-.011	4	-8.699e-4	4	NC	1	2343.1	4





Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.005	4	-8.699e-4	4	NC	1	4840.86	4
377		18	max	0	1	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.002	4	-8.699e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-8.699e-4	4	NC	1	NC	1
381	M10	1	max	.007	1	.008	2	0	12	2.857e-3	4	NC	1	NC	2
382			min	-.008	3	-.013	3	-.973	4	2.042e-5	12	9894.859	2	79.65	4
383		2	max	.007	1	.006	2	0	12	2.915e-3	4	NC	1	NC	2
384			min	-.007	3	-.013	3	-.894	4	1.939e-5	12	NC	1	86.707	4
385		3	max	.006	1	.005	2	0	12	2.972e-3	4	NC	1	NC	2
386			min	-.007	3	-.013	3	-.815	4	1.836e-5	12	NC	1	95.084	4
387		4	max	.006	1	.004	2	0	12	3.03e-3	4	NC	1	NC	2
388			min	-.007	3	-.012	3	-.737	4	1.732e-5	12	NC	1	105.126	4
389		5	max	.006	1	.003	2	0	12	3.088e-3	4	NC	1	NC	2
390			min	-.006	3	-.012	3	-.661	4	1.629e-5	12	NC	1	117.298	4
391		6	max	.005	1	.001	2	0	12	3.145e-3	4	NC	1	NC	2
392			min	-.006	3	-.012	3	-.586	4	1.525e-5	12	NC	1	132.248	4
393		7	max	.005	1	0	2	0	12	3.203e-3	4	NC	1	NC	1
394			min	-.005	3	-.011	3	-.514	4	1.422e-5	12	NC	1	150.892	4
395		8	max	.004	1	0	2	0	12	3.261e-3	4	NC	1	NC	1
396			min	-.005	3	-.011	3	-.444	4	1.318e-5	12	NC	1	174.566	4
397		9	max	.004	1	-.002	2	0	12	3.318e-3	4	NC	1	NC	1
398			min	-.004	3	-.01	3	-.377	4	1.215e-5	12	NC	1	205.28	4
399		10	max	.004	1	-.002	2	0	12	3.376e-3	4	NC	1	NC	1
400			min	-.004	3	-.01	3	-.315	4	1.112e-5	12	NC	1	246.17	4
401		11	max	.003	1	-.002	15	0	12	3.433e-3	4	NC	1	NC	1
402			min	-.004	3	-.009	3	-.256	4	1.008e-5	12	NC	1	302.362	4
403		12	max	.003	1	-.002	15	0	12	3.491e-3	4	NC	1	NC	1
404			min	-.003	3	-.008	3	-.202	4	9.047e-6	12	NC	1	382.706	4
405		13	max	.002	1	-.002	15	0	12	3.549e-3	4	NC	1	NC	1
406			min	-.003	3	-.008	4	-.154	4	8.012e-6	12	NC	1	503.602	4
407		14	max	.002	1	-.002	15	0	12	3.606e-3	4	NC	1	NC	1
408			min	-.002	3	-.007	4	-.111	4	6.978e-6	12	NC	1	698.331	4
409		15	max	.002	1	-.002	15	0	12	3.664e-3	4	NC	1	NC	1
410			min	-.002	3	-.006	4	-.074	4	5.944e-6	12	NC	1	1043.366	4
411		16	max	.001	1	-.001	15	0	12	3.722e-3	4	NC	1	NC	1
412			min	-.001	3	-.005	4	-.044	4	4.909e-6	12	NC	1	1749.167	4
413		17	max	0	1	0	15	0	12	3.779e-3	4	NC	1	NC	1
414			min	0	3	-.004	4	-.022	4	3.875e-6	12	NC	1	3590.591	4
415		18	max	0	1	0	15	0	12	3.837e-3	4	NC	1	NC	1
416			min	0	3	-.002	4	-.007	4	2.841e-6	12	NC	1	NC	1
417		19	max	0	1	0	1	0	1	3.895e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	1.806e-6	12	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	-3.503e-7	12	NC	1	NC	1
420			min	0	1	0	1	0	1	-8.452e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.021	4	-2.332e-6	12	NC	1	NC	1
422			min	0	2	-.003	4	0	12	-4.18e-5	1	NC	1	NC	1
423		3	max	0	3	-.002	15	.04	4	7.668e-4	5	NC	1	NC	1
424			min	0	2	-.006	4	0	12	-7.818e-5	1	NC	1	7454.887	4
425		4	max	.001	3	-.002	15	.058	4	1.568e-3	4	NC	1	NC	1
426			min	0	2	-.01	4	0	12	-1.146e-4	1	NC	1	5599.909	4
427		5	max	.002	3	-.003	15	.074	4	2.372e-3	4	NC	1	NC	1
428			min	-.001	2	-.013	4	0	12	-1.509e-4	1	8243.393	4	4785.446	4
429		6	max	.002	3	-.004	15	.089	4	3.177e-3	4	NC	5	NC	1
430			min	-.001	2	-.016	4	0	12	-1.873e-4	1	6699.357	4	4418.855	4
431		7	max	.002	3	-.004	15	.103	4	3.981e-3	4	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432		min	-.002	2	-.018	4	0	10	-2.237e-4	1	5768.609	4	4315.447	4
433	8	max	.003	3	-.005	15	.116	4	4.786e-3	4	NC	5	NC	1
434		min	-.002	2	-.02	4	0	1	-2.601e-4	1	5194.924	4	4414.291	4
435	9	max	.003	3	-.005	15	.128	4	5.59e-3	4	NC	5	NC	1
436		min	-.002	2	-.022	4	0	1	-2.965e-4	1	4857.653	4	4712.604	4
437	10	max	.003	3	-.006	15	.139	4	6.394e-3	4	NC	5	NC	1
438		min	-.003	2	-.022	4	-.001	1	-3.328e-4	1	4698.434	4	5254.193	4
439	11	max	.004	3	-.006	15	.149	4	7.199e-3	4	NC	5	NC	1
440		min	-.003	2	-.023	4	-.002	1	-3.692e-4	1	4693.931	4	6146.438	4
441	12	max	.004	3	-.005	15	.159	4	8.003e-3	4	NC	5	NC	1
442		min	-.003	2	-.022	4	-.002	1	-4.056e-4	1	4846.893	4	7619.194	4
443	13	max	.005	3	-.005	15	.169	4	8.808e-3	4	NC	5	NC	1
444		min	-.004	2	-.021	4	-.003	1	-4.42e-4	1	5188.366	4	NC	1
445	14	max	.005	3	-.005	15	.179	4	9.612e-3	4	NC	5	NC	1
446		min	-.004	2	-.019	4	-.004	1	-4.784e-4	1	5793.604	4	NC	1
447	15	max	.005	3	-.004	15	.189	4	1.042e-2	4	NC	3	NC	1
448		min	-.004	2	-.016	4	-.006	1	-5.147e-4	1	6828.716	4	NC	1
449	16	max	.006	3	-.003	15	.2	4	1.122e-2	4	NC	1	NC	1
450		min	-.004	2	-.013	4	-.007	1	-5.511e-4	1	8695.476	4	NC	1
451	17	max	.006	3	-.002	15	.212	4	1.203e-2	4	NC	1	NC	1
452		min	-.005	2	-.01	4	-.009	1	-5.875e-4	1	NC	1	NC	1
453	18	max	.007	3	-.002	15	.224	4	1.283e-2	4	NC	1	NC	2
454		min	-.005	2	-.006	4	-.011	1	-6.239e-4	1	NC	1	9279.418	1
455	19	max	.007	3	0	10	.238	4	1.363e-2	4	NC	1	NC	2
456		min	-.005	2	-.002	1	-.013	1	-6.603e-4	1	NC	1	7741.692	1
457	M12	1	max	.003	1	.005	.013	1	-1.32e-5	12	NC	1	NC	3
458		min	0	12	-.007	3	-.238	4	-7.454e-4	4	NC	1	104.017	4
459	2	max	.002	1	.005	2	.012	1	-1.32e-5	12	NC	1	NC	3
460		min	0	12	-.007	3	-.219	4	-7.454e-4	4	NC	1	113.151	4
461	3	max	.002	1	.004	2	.011	1	-1.32e-5	12	NC	1	NC	3
462		min	0	12	-.006	3	-.2	4	-7.454e-4	4	NC	1	124.019	4
463	4	max	.002	1	.004	2	.01	1	-1.32e-5	12	NC	1	NC	3
464		min	0	12	-.006	3	-.181	4	-7.454e-4	4	NC	1	137.072	4
465	5	max	.002	1	.004	2	.009	1	-1.32e-5	12	NC	1	NC	3
466		min	0	12	-.006	3	-.162	4	-7.454e-4	4	NC	1	152.923	4
467	6	max	.002	1	.004	2	.008	1	-1.32e-5	12	NC	1	NC	3
468		min	0	12	-.005	3	-.144	4	-7.454e-4	4	NC	1	172.42	4
469	7	max	.002	1	.003	2	.007	1	-1.32e-5	12	NC	1	NC	3
470		min	0	12	-.005	3	-.126	4	-7.454e-4	4	NC	1	196.77	4
471	8	max	.002	1	.003	2	.006	1	-1.32e-5	12	NC	1	NC	3
472		min	0	12	-.004	3	-.109	4	-7.454e-4	4	NC	1	227.732	4
473	9	max	.001	1	.003	2	.005	1	-1.32e-5	12	NC	1	NC	2
474		min	0	12	-.004	3	-.093	4	-7.454e-4	4	NC	1	267.955	4
475	10	max	.001	1	.003	2	.004	1	-1.32e-5	12	NC	1	NC	2
476		min	0	12	-.004	3	-.077	4	-7.454e-4	4	NC	1	321.577	4
477	11	max	.001	1	.002	2	.004	1	-1.32e-5	12	NC	1	NC	2
478		min	0	12	-.003	3	-.063	4	-7.454e-4	4	NC	1	395.37	4
479	12	max	.001	1	.002	2	.003	1	-1.32e-5	12	NC	1	NC	2
480		min	0	12	-.003	3	-.05	4	-7.454e-4	4	NC	1	501.049	4
481	13	max	0	1	.002	2	.002	1	-1.32e-5	12	NC	1	NC	1
482		min	0	12	-.002	3	-.038	4	-7.454e-4	4	NC	1	660.358	4
483	14	max	0	1	.001	2	.002	1	-1.32e-5	12	NC	1	NC	1
484		min	0	12	-.002	3	-.027	4	-7.454e-4	4	NC	1	917.533	4
485	15	max	0	1	.001	2	.001	1	-1.32e-5	12	NC	1	NC	1
486		min	0	12	-.002	3	-.018	4	-7.454e-4	4	NC	1	1374.545	4
487	16	max	0	1	0	2	0	1	-1.32e-5	12	NC	1	NC	1
488		min	0	12	-.001	3	-.011	4	-7.454e-4	4	NC	1	2313.286	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-1.32e-5	12	NC	1	NC	1
490			min	0	12	0	3	-0.005	4	-7.454e-4	4	NC	1	4778.943	4
491		18	max	0	1	0	2	0	1	-1.32e-5	12	NC	1	NC	1
492			min	0	12	0	3	-.002	4	-7.454e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-1.32e-5	12	NC	1	NC	1
494			min	0	1	0	1	0	1	-7.454e-4	4	NC	1	NC	1
495	M1	1	max	.009	3	.162	2	1.035	4	1.316e-2	1	NC	1	NC	1
496			min	-.004	2	-.03	3	0	12	-2.069e-2	3	NC	1	NC	1
497		2	max	.009	3	.08	1	1	4	1.033e-2	4	NC	5	NC	1
498			min	-.004	2	-.013	3	-.01	1	-1.027e-2	3	1620.276	2	8154.439	5
499		3	max	.009	3	.013	3	.964	4	1.769e-2	4	NC	5	NC	2
500			min	-.004	2	-.011	2	-.014	1	-3.008e-4	1	781.847	2	4499.566	5
501		4	max	.009	3	.057	3	.926	4	1.536e-2	4	NC	15	NC	2
502			min	-.004	2	-.111	2	-.013	1	-4.179e-3	3	494.893	2	3267.96	5
503		5	max	.008	3	.112	3	.887	4	1.303e-2	4	9888.605	15	NC	1
504			min	-.004	2	-.216	2	-.009	1	-8.251e-3	3	357.006	1	2649.039	5
505		6	max	.008	3	.172	3	.847	4	1.38e-2	1	7801.805	15	NC	1
506			min	-.004	2	-.318	1	-.004	1	-1.232e-2	3	280.38	1	2272.759	5
507		7	max	.008	3	.229	3	.807	4	1.85e-2	1	6571.235	15	NC	1
508			min	-.004	2	-.41	1	0	3	-1.64e-2	3	235.246	1	1998.653	4
509		8	max	.008	3	.277	3	.765	4	2.32e-2	1	5843.791	15	NC	1
510			min	-.004	2	-.483	1	0	12	-2.047e-2	3	208.596	1	1791.765	4
511		9	max	.008	3	.309	3	.722	4	2.562e-2	1	5463.678	15	NC	1
512			min	-.004	2	-.53	1	0	1	-2.074e-2	3	194.718	1	1654.548	4
513		10	max	.008	3	.32	3	.675	4	2.651e-2	1	5347.623	15	NC	1
514			min	-.004	2	-.545	1	0	12	-1.846e-2	3	190.566	1	1613.544	4
515		11	max	.007	3	.312	3	.624	4	2.816e-2	2	5463.429	15	NC	1
516			min	-.004	2	-.529	1	0	12	-1.619e-2	3	195.027	1	1648.407	4
517		12	max	.007	3	.286	3	.569	4	2.71e-2	2	5843.207	15	NC	1
518			min	-.004	2	-.482	1	-.001	1	-1.373e-2	3	209.545	1	1766.253	4
519		13	max	.007	3	.244	3	.508	4	2.176e-2	2	6570.101	15	NC	1
520			min	-.004	2	-.407	1	0	1	-1.098e-2	3	237.565	1	2097.599	4
521		14	max	.007	3	.189	3	.442	4	1.642e-2	2	7799.724	15	NC	1
522			min	-.003	2	-.313	1	0	12	-8.23e-3	3	285.333	1	2833.175	4
523		15	max	.007	3	.128	3	.374	4	1.108e-2	2	9884.782	15	NC	1
524			min	-.003	2	-.208	1	0	12	-5.482e-3	3	367.161	1	4587.604	4
525		16	max	.006	3	.065	3	.309	4	1.079e-2	4	NC	15	NC	1
526			min	-.003	2	-.102	1	0	12	-2.734e-3	3	517.633	1	NC	1
527		17	max	.006	3	.005	3	.248	4	1.208e-2	4	NC	5	NC	2
528			min	-.003	2	-.006	2	0	12	1.187e-5	12	836.561	1	9533.673	1
529		18	max	.006	3	.08	1	.194	4	9.011e-3	2	NC	5	NC	1
530			min	-.003	2	-.05	3	0	12	-3.193e-3	3	1762.642	1	NC	1
531		19	max	.006	3	.156	1	.148	4	1.787e-2	2	NC	1	NC	1
532			min	-.003	2	-.101	3	-.001	1	-6.507e-3	3	NC	1	NC	1
533	M5	1	max	.027	3	.34	2	1.035	4	0	1	NC	1	NC	1
534			min	-.019	2	-.026	3	0	1	-9.747e-6	4	NC	1	NC	1
535		2	max	.027	3	.164	1	1.008	4	9.078e-3	4	NC	5	NC	1
536			min	-.019	2	-.008	3	0	1	0	1	771.115	2	6030.126	4
537		3	max	.027	3	.041	3	.974	4	1.795e-2	4	NC	15	NC	1
538			min	-.019	2	-.036	2	0	1	0	1	362.086	2	3566.54	4
539		4	max	.026	3	.149	3	.935	4	1.462e-2	4	6861.793	15	NC	1
540			min	-.018	2	-.274	2	0	1	0	1	221.334	2	2787.746	4
541		5	max	.026	3	.296	3	.893	4	1.13e-2	4	4786.947	15	NC	1
542			min	-.018	2	-.534	1	0	1	0	1	154.84	1	2422.273	4
543		6	max	.025	3	.463	3	.85	4	7.976e-3	4	3676.477	15	NC	1
544			min	-.018	2	-.797	1	0	1	0	1	118.871	1	2200.009	4
545		7	max	.025	3	.625	3	.806	4	4.652e-3	4	3036.624	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.017	2	-1.036	1	0	1	0	1	98.131	1	2016.93	4
547		8	max	.024	3	.762	3	.764	4	1.328e-3	4	2665.345	15	NC	1
548			min	-.017	2	-1.228	1	0	1	0	1	86.091	1	1822.187	4
549		9	max	.024	3	.85	3	.722	4	0	1	2475.004	15	NC	1
550			min	-.016	2	-1.349	1	0	1	-5.859e-6	5	79.919	1	1648.279	4
551		10	max	.023	3	.882	3	.674	4	0	1	2417.616	15	NC	1
552			min	-.016	2	-1.39	1	0	1	-5.651e-6	5	78.087	1	1626.409	4
553		11	max	.022	3	.86	3	.623	4	0	1	2475.104	15	NC	1
554			min	-.016	2	-1.348	1	0	1	-5.443e-6	5	80.064	1	1672.588	4
555		12	max	.022	3	.785	3	.571	4	8.441e-4	4	2665.589	15	NC	1
556			min	-.016	2	-1.223	1	0	1	0	1	86.568	1	1730.609	4
557		13	max	.021	3	.665	3	.51	4	2.958e-3	4	3037.136	15	NC	1
558			min	-.015	2	-1.025	1	0	1	0	1	99.373	1	2048.764	4
559		14	max	.021	3	.513	3	.441	4	5.072e-3	4	3677.495	15	NC	1
560			min	-.015	2	-.778	1	0	1	0	1	121.681	1	2935.086	4
561		15	max	.02	3	.344	3	.369	4	7.186e-3	4	4788.986	15	NC	1
562			min	-.015	2	-.51	1	0	1	0	1	160.979	1	5797.712	4
563		16	max	.019	3	.173	3	.299	4	9.3e-3	4	6866.103	15	NC	1
564			min	-.015	2	-.247	1	0	1	0	1	235.861	1	NC	1
565		17	max	.019	3	.014	3	.237	4	1.141e-2	4	NC	15	NC	1
566			min	-.014	2	-.018	2	0	1	0	1	400.456	1	NC	1
567		18	max	.019	3	.167	1	.186	4	5.773e-3	4	NC	5	NC	1
568			min	-.014	2	-.122	3	0	1	0	1	877.891	1	NC	1
569		19	max	.019	3	.319	1	.149	4	0	1	NC	1	NC	1
570			min	-.014	2	-.245	3	0	1	-5.737e-6	4	NC	1	NC	1
571	M9	1	max	.009	3	.162	2	1.035	4	2.069e-2	3	NC	1	NC	1
572			min	-.004	2	-.03	3	-.001	1	-1.316e-2	1	NC	1	NC	1
573		2	max	.009	3	.08	1	1.006	4	1.027e-2	3	NC	5	NC	1
574			min	-.004	2	-.013	3	0	12	-6.327e-3	1	1620.276	2	6427.583	4
575		3	max	.009	3	.013	3	.972	4	1.789e-2	4	NC	5	NC	2
576			min	-.004	2	-.011	2	0	12	4.781e-6	10	781.847	2	3737.116	4
577		4	max	.009	3	.057	3	.934	4	1.4e-2	5	NC	15	NC	2
578			min	-.004	2	-.111	2	0	12	-4.399e-3	1	494.893	2	2866.034	4
579		5	max	.008	3	.112	3	.893	4	1.055e-2	5	9840.069	15	NC	1
580			min	-.004	2	-.216	2	0	12	-9.099e-3	1	357.006	1	2445.319	4
581		6	max	.008	3	.172	3	.85	4	1.232e-2	3	7765.156	15	NC	1
582			min	-.004	2	-.318	1	0	12	-1.38e-2	1	280.38	1	2189.304	4
583		7	max	.008	3	.229	3	.807	4	1.64e-2	3	6541.38	15	NC	1
584			min	-.004	2	-.41	1	0	1	-1.85e-2	1	235.246	1	1993.452	4
585		8	max	.008	3	.277	3	.764	4	2.047e-2	3	5817.856	15	NC	1
586			min	-.004	2	-.483	1	-.001	1	-2.32e-2	1	208.596	1	1808.153	4
587		9	max	.008	3	.309	3	.722	4	2.074e-2	3	5439.744	15	NC	1
588			min	-.004	2	-.53	1	0	12	-2.562e-2	1	194.718	1	1648.572	4
589		10	max	.008	3	.32	3	.675	4	1.846e-2	3	5324.28	15	NC	1
590			min	-.004	2	-.545	1	0	1	-2.651e-2	1	190.566	1	1614.715	4
591		11	max	.007	3	.312	3	.623	4	1.619e-2	3	5439.515	15	NC	1
592			min	-.004	2	-.529	1	0	1	-2.816e-2	2	195.027	1	1656.189	4
593		12	max	.007	3	.286	3	.57	4	1.373e-2	3	5817.444	15	NC	1
594			min	-.004	2	-.482	1	0	12	-2.71e-2	2	209.545	1	1750.403	4
595		13	max	.007	3	.244	3	.508	4	1.098e-2	3	6540.795	15	NC	1
596			min	-.004	2	-.407	1	0	12	-2.176e-2	2	237.565	1	2097.986	4
597		14	max	.007	3	.189	3	.44	4	8.23e-3	3	7764.356	15	NC	1
598			min	-.003	2	-.313	1	-.003	1	-1.642e-2	2	285.333	1	2942.447	5
599		15	max	.007	3	.128	3	.369	4	6.832e-3	5	9838.952	15	NC	1
600			min	-.003	2	-.208	1	-.008	1	-1.108e-2	2	367.161	1	5191.418	5
601		16	max	.006	3	.065	3	.301	4	9.198e-3	5	NC	15	NC	1
602			min	-.003	2	-.102	1	-.012	1	-5.863e-3	1	517.633	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 4, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.006	3	.005	3	.239	4	1.159e-2	4	NC	5	NC	2
604		min	-.003	2	-.006	2	-.013	1	-8.458e-4	1	836.561	1	9533.673	1
605	18	max	.006	3	.08	1	.189	4	5.527e-3	5	NC	5	NC	1
606		min	-.003	2	-.05	3	-.009	1	-9.011e-3	2	1762.642	1	NC	1
607	19	max	.006	3	.156	1	.149	4	6.507e-3	3	NC	1	NC	1
608		min	-.003	2	-.101	3	0	12	-1.787e-2	2	NC	1	NC	1



Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

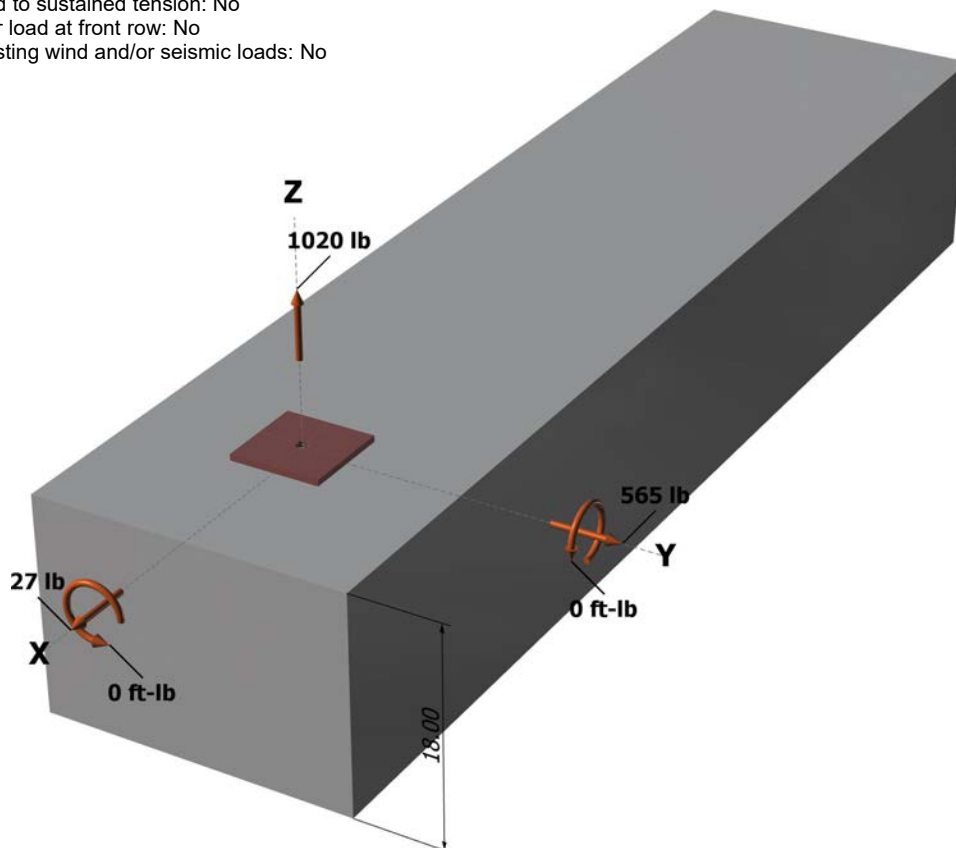
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
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Company:	Schletter, Inc.	Date:	8/1/2016
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Address:			
Phone:			
E-mail:			

<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 [www.strongtie.com](http://www.strongtie.com)



# Anchor Designer™ Software Version 2.4.6025.0

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Address:			
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1020.0	27.0	565.0	565.6
Sum	1020.0	27.0	565.0	565.6

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1020  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

#### Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cby} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

#### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

#### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

#### Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cby} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cby}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a ; k_{cp} N_{cb}] = \phi \min[k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} ; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-40 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	1020	6071	0.17	Pass	
Concrete breakout	1020	5710	0.18	Pass	
<b>Adhesive</b>	<b>1020</b>	<b>5365</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
<b>Steel</b>	<b>566</b>	<b>3156</b>	<b>0.18</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	565	3934	0.14	Pass	
T Concrete breakout x+	27	3018	0.01	Pass	
Concrete breakout y+	27	8508	0.00	Pass	
Concrete breakout x+	565	6875	0.08	Pass	
Concrete breakout, combined	-	-	0.14	Pass	
Pryout	566	12298	0.05	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status
Sec. D.7.1	0.19	0.00	19.0 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





Anchor Designer™  
Software  
Version 2.4.6025.0

Company:	Schletter, Inc.	Date:	8/1/2016
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 21-31 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $c_{ac}$  (inch): 9.67  
 $c_{min}$  (inch): 1.75  
 $s_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

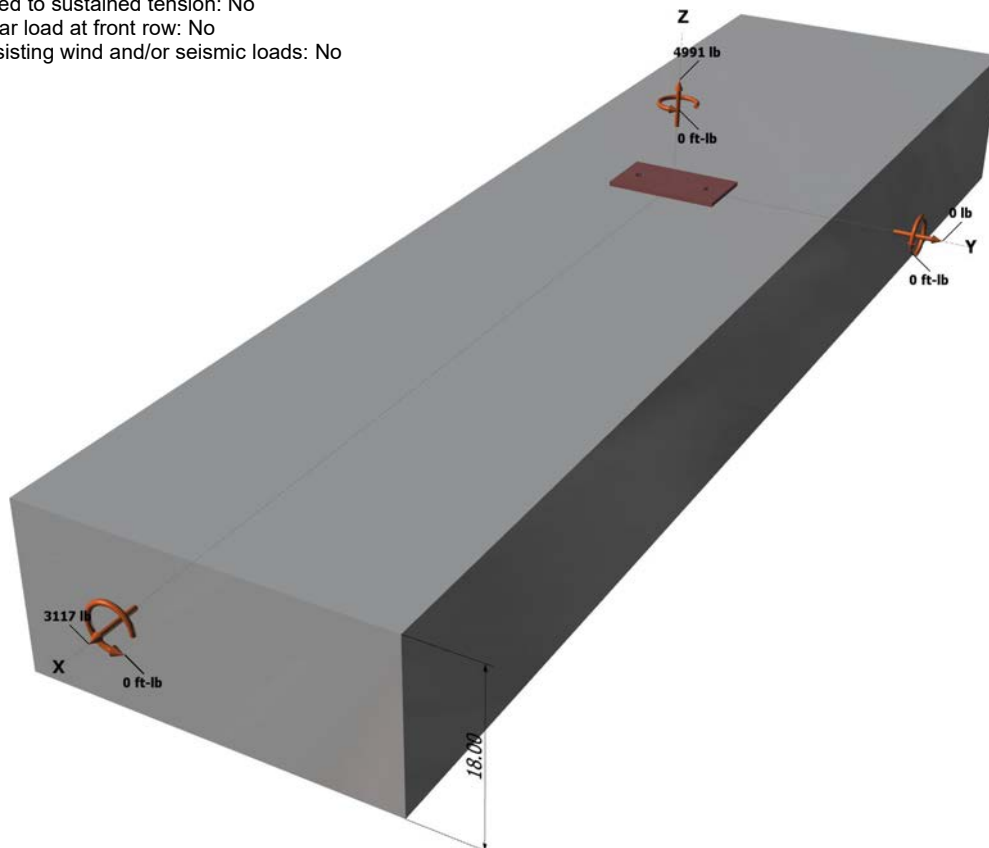
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

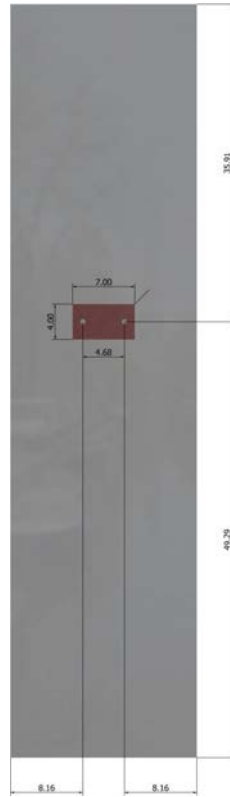
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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2495.5	1558.5	0.0	1558.5
2	2495.5	1558.5	0.0	1558.5
Sum	4991.0	3117.0	0.0	3117.0

Maximum concrete compression strain (%): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 4991

Resultant compression force (lb): 0

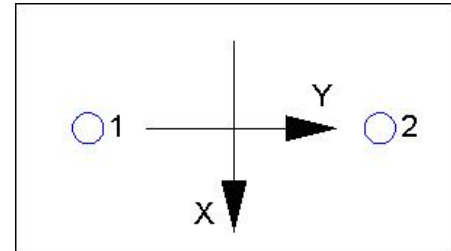
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00

Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00

Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00

Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
378.00	324.00	1.000	0.972	1.00	1.000	12492	0.65	9208

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,crf} \text{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{\text{short-term}}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
378.00	648.00	1.000	0.836	1.000	1.000	15593	0.70	5323

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.16	8744

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
299.64	299.64	1.000	1.000	1.000	8744	0.70	12241

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min |k_{cp} N_{ag}; k_{cp} N_{cbg}| = \phi \min |k_{cp} (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp} (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b| \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
378.00	324.00	1.000	0.972	1.000	1.000	12492	14166	0.70

$$\phi V_{cp} = 19833$$

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	2496	6071	0.41	Pass
Concrete breakout	4991	9208	0.54	Pass
<b>Adhesive</b>	<b>4991</b>	<b>8093</b>	<b>0.62</b>	<b>Pass (Governs)</b>
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	1559	3156	0.49	Pass
<b>T Concrete breakout x+</b>	<b>3117</b>	<b>5323</b>	<b>0.59</b>	<b>Pass (Governs)</b>

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Concrete breakout y-	1559	12241	0.13	Pass (Governs)
Pryout	3117	19833	0.16	Pass

Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.3	0.62	0.59	120.2 %	1.2	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.