

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.05	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	111 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	1.913 k-ft
$M_z$ =	0.260 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>91%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.364 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.710 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>98%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

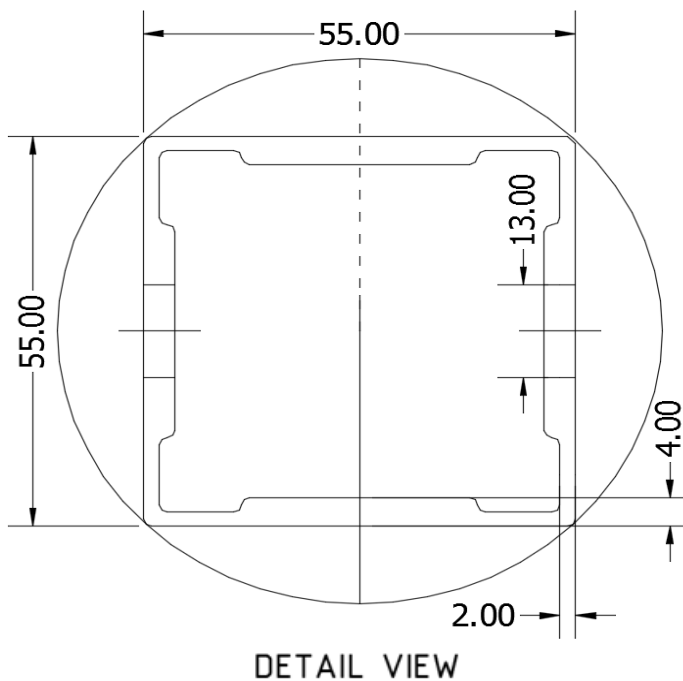
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.473 k-ft
$P_n$ =	0.606 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>36%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

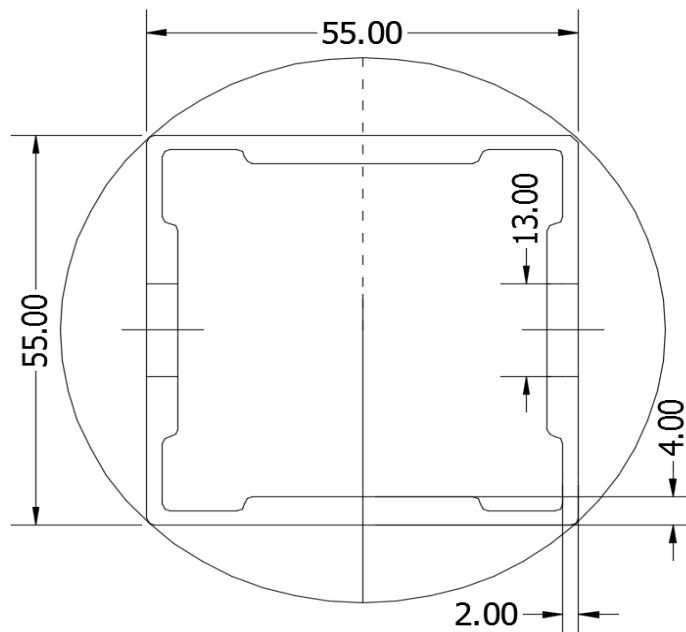
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.007 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>28%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	55.91 in
$\Phi F_{ty \text{ AXIAL}}$ =	15.92 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.516 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	15.642 k
Utilization =	<u>23%</u>



#### 5. FOUNDATION DESIGN CALCULATIONS

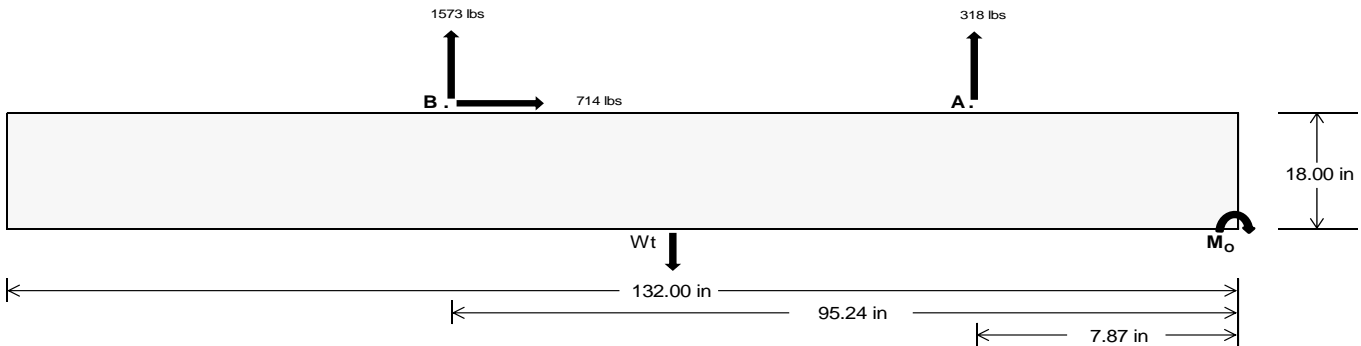
##### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	Maximum	Front	Rear
Tensile Load =		<u>1398.00</u>	<u>6833.10</u> k
Compressive Load =		<u>4518.78</u>	<u>5281.16</u> k
Lateral Load =		<u>316.10</u>	<u>3094.41</u> k
Moment (Weak Axis) =		<u>0.63</u>	<u>0.33</u> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 165117.7$  in-lbs  
Resisting Force Required = 2501.78 lbs  
S.F. = 1.67  
Weight Required = 4169.64 lbs  
Minimum Width = 36 in  
Weight Provided = 7177.50 lbs

### Sliding

Force = 713.84 lbs  
Friction = 0.4  
Weight Required = 1784.59 lbs  
Resisting Weight = 7177.50 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 713.84 lbs  
Cohesion = 130 psf  
Area = 33.00 ft<sup>2</sup>  
Resisting = 3588.75 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 132in long x 36in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 36in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 36in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(3 \text{ ft}) =$$

### Ballast Width

36 in	37 in	38 in	39 in
7178 lbs	7377 lbs	7576 lbs	7776 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in	36 in	37 in	38 in	39 in
$F_A$	1437 lbs	1437 lbs	1437 lbs	1437 lbs	1768 lbs	1768 lbs	1768 lbs	1768 lbs	2284 lbs	2284 lbs	2284 lbs	2284 lbs	-636 lbs	-636 lbs	-636 lbs	-636 lbs
$F_B$	1463 lbs	1463 lbs	1463 lbs	1463 lbs	2138 lbs	2138 lbs	2138 lbs	2138 lbs	2580 lbs	2580 lbs	2580 lbs	2580 lbs	-3145 lbs	-3145 lbs	-3145 lbs	-3145 lbs
$F_V$	160 lbs	160 lbs	160 lbs	160 lbs	1268 lbs	1268 lbs	1268 lbs	1268 lbs	1058 lbs	1058 lbs	1058 lbs	1058 lbs	-1428 lbs	-1428 lbs	-1428 lbs	-1428 lbs
$P_{total}$	10077 lbs	10276 lbs	10475 lbs	10675 lbs	11084 lbs	11284 lbs	11483 lbs	11682 lbs	12042 lbs	12241 lbs	12440 lbs	12640 lbs	525 lbs	645 lbs	765 lbs	884 lbs
$M$	3634 lbs-ft	3634 lbs-ft	3634 lbs-ft	3634 lbs-ft	5258 lbs-ft	5258 lbs-ft	5258 lbs-ft	5258 lbs-ft	6366 lbs-ft	6366 lbs-ft	6366 lbs-ft	6366 lbs-ft	2440 lbs-ft	2440 lbs-ft	2440 lbs-ft	2440 lbs-ft
$e$	0.36 ft	0.35 ft	0.35 ft	0.34 ft	0.47 ft	0.47 ft	0.46 ft	0.45 ft	0.53 ft	0.52 ft	0.51 ft	0.50 ft	4.65 ft	3.78 ft	3.19 ft	2.76 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	245.3 psf	244.5 psf	243.8 psf	243.1 psf	249.0 psf	248.1 psf	247.3 psf	246.6 psf	259.7 psf	258.5 psf	257.4 psf	256.4 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	365.4 psf	361.4 psf	357.6 psf	354.0 psf	422.8 psf	417.2 psf	412.0 psf	407.0 psf	470.1 psf	463.3 psf	456.8 psf	450.7 psf	136.6 psf	81.2 psf	69.7 psf	66.2 psf

Maximum Bearing Pressure = 470 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

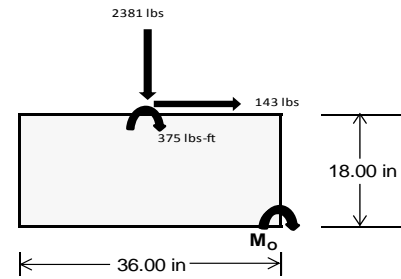
### Overturning Check

$M_o = 2981.7 \text{ ft-lbs}$   
 Resisting Force Required = 1987.82 lbs  
 S.F. = 1.67  
 Weight Required = 3313.04 lbs  
 Minimum Width = **36 in**  
 Weight Provided = 7177.50 lbs

*A minimum 132in long x 36in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	36 in			36 in			36 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	252 lbs	595 lbs	202 lbs	828 lbs	2381 lbs	790 lbs	91 lbs	174 lbs	42 lbs
$F_v$	198 lbs	195 lbs	200 lbs	147 lbs	143 lbs	155 lbs	198 lbs	196 lbs	199 lbs
$P_{total}$	9138 lbs	9481 lbs	9088 lbs	9287 lbs	10839 lbs	9248 lbs	2689 lbs	2772 lbs	2640 lbs
$M$	786 lbs-ft	778 lbs-ft	792 lbs-ft	592 lbs-ft	589 lbs-ft	618 lbs-ft	785 lbs-ft	777 lbs-ft	787 lbs-ft
$e$	0.09 ft	0.08 ft	0.09 ft	0.06 ft	0.05 ft	0.07 ft	0.29 ft	0.28 ft	0.30 ft
$L/6$	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft	0.50 ft
$f_{min}$	229.2 psf	240.1 psf	227.4 psf	245.6 psf	292.8 psf	242.8 psf	33.9 psf	36.9 psf	32.3 psf
$f_{max}$	324.6 psf	334.5 psf	323.4 psf	317.3 psf	364.2 psf	317.7 psf	129.0 psf	131.1 psf	127.7 psf



Maximum Bearing Pressure = 364 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 36in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 36in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.799 k
Allowable Uplift =	1.214 k
Utilization =	<u>66%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.638 k
Allowable Uplift =	4.357 k
Utilization =	<u>61%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.476 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>47%</u>

#### Rear Strut

Maximum Axial Load =	4.693 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>63%</u>

#### Diagonal Strut

Maximum Axial Load =	2.139 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>29%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	40.12 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.802 in
	<u>0.491 ≤ 0.802, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 111 \text{ in}$$

$$J = 0.432$$

$$307.078$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.6 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 111$$

$$J = 0.432$$

$$195.283$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.8$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$\begin{aligned} Rb/t &= 18.1 \\ S1 &= \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2 \\ S1 &= 1.1 \\ S2 &= C_t \\ S2 &= 141.0 \\ \phi F_L &= \phi b [Bt - Dt \sqrt{(Rb/t)}] \\ \phi F_L &= 31.1 \text{ ksi} \end{aligned}$$

### 3.4.18

$$\begin{aligned} h/t &= 7.4 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 35.2 \\ m &= 0.68 \\ C_0 &= 41.067 \\ Cc &= 43.717 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 73.8 \\ \phi F_L &= 1.3\phi y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L St &= 29.4 \text{ ksi} \\ I_x &= 984962 \text{ mm}^4 \\ &= 2.366 \text{ in}^4 \\ y &= 43.717 \text{ mm} \\ S_x &= 1.375 \text{ in}^3 \\ M_{max} St &= 3.363 \text{ k-ft} \end{aligned}$$

### 3.4.16.1 N/A for Weak Direction

### 3.4.18

$$\begin{aligned} h/t &= 16.2 \\ S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\ S1 &= 36.9 \\ m &= 0.65 \\ C_0 &= 40 \\ Cc &= 40 \\ S2 &= \frac{k_1 Bbr}{mDbr} \\ S2 &= 77.3 \\ \phi F_L &= 1.3\phi y Fcy \\ \phi F_L &= 43.2 \text{ ksi} \\ \phi F_L Wk &= 33.3 \text{ ksi} \\ I_y &= 923544 \text{ mm}^4 \\ &= 2.219 \text{ in}^4 \\ x &= 40 \text{ mm} \\ S_y &= 1.409 \text{ in}^3 \\ M_{max} Wk &= 3.904 \text{ k-ft} \end{aligned}$$

### Compression

### 3.4.9

$$\begin{aligned} b/t &= 16.2 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 7.4 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 18.1 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi c [Bt - Dt \sqrt{(Rb/t)}] \\ \phi F_L &= 31.09 \text{ ksi} \\ \phi F_L &= 31.09 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{max} &= 58.55 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \text{ in} \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.29339$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76107$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 15.9235 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

**3.4.10**

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 15.92 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 16.39 \text{ kips}$$

**APPENDIX B****B.1**

*The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.*



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-54.031	-54.031	0	0
2	M14	Y	-54.031	-54.031	0	0
3	M15	Y	-54.031	-54.031	0	0
4	M16	Y	-54.031	-54.031	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-103.443	-103.443	0	0
2	M14	y	-103.443	-103.443	0	0
3	M15	y	-162.554	-162.554	0	0
4	M16	y	-162.554	-162.554	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	236.442	236.442	0	0
2	M14	y	181.272	181.272	0	0
3	M15	y	98.517	98.517	0	0
4	M16	y	98.517	98.517	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 20° 150mph 30psf 9.25ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	83.742	1	1337.635	3	162.18	1	.016	2	.261	1	1.603	1
20			min	4.499	12	-839.654	1	-95.065	14	0	3	.006	12	-2.473	3
21		11	max	83.742	1	691.564	1	-3.84	12	.016	2	.112	1	.816	1
22			min	4.499	12	-1099.8	3	-127.628	1	0	3	.001	12	-1.221	3
23		12	max	83.742	1	543.475	1	-2.584	12	.016	2	.042	4	.182	2
24			min	4.499	12	-861.966	3	-93.076	1	0	3	-.004	3	-.213	3
25		13	max	83.742	1	395.385	1	-1.329	12	.016	2	.019	5	.551	3
26			min	4.499	12	-624.132	3	-58.524	1	0	3	-.079	1	-.301	1
27		14	max	83.742	1	247.295	1	.019	3	.016	2	0	15	1.07	3
28			min	2.956	15	-386.297	3	-24.136	4	0	3	-.121	1	-.632	1
29		15	max	83.742	1	99.205	1	10.581	1	.016	2	-.004	12	1.345	3
30			min	-6.02	5	-148.463	3	-16.958	5	0	3	-.128	1	-.81	1
31		16	max	83.742	1	89.372	3	45.133	1	.016	2	-.003	12	1.376	3
32			min	-16.57	5	-48.885	1	-15.015	5	0	3	-.1	1	-.836	1
33		17	max	83.742	1	327.206	3	79.686	1	.016	2	.001	3	1.161	3
34			min	-27.12	5	-196.975	1	-13.072	5	0	3	-.058	4	-.709	1
35		18	max	83.742	1	565.041	3	114.238	1	.016	2	.064	1	.703	3
36			min	-37.671	5	-345.065	1	-11.129	5	0	3	-.063	5	-.431	1
37		19	max	83.742	1	802.875	3	148.79	1	.016	2	.199	1	0	1
38			min	-48.221	5	-493.154	1	-9.186	5	0	3	-.073	5	0	3
39	M14	1	max	53.513	4	529.204	1	-6.381	12	.011	3	.23	1	0	1
40			min	1.944	12	-633.617	3	-153.704	1	-.013	2	.012	12	0	3
41		2	max	42.963	4	381.114	1	-5.126	12	.011	3	.124	4	.558	3
42			min	1.944	12	-452.485	3	-119.152	1	-.013	2	.006	12	-.468	1
43		3	max	40.86	1	233.024	1	-3.87	12	.011	3	.069	5	.93	3
44			min	1.944	12	-271.353	3	-84.599	1	-.013	2	-.015	1	-.783	1
45		4	max	40.86	1	84.942	2	-2.614	12	.011	3	.038	5	1.116	3
46			min	1.944	12	-90.221	3	-50.047	1	-.013	2	-.085	1	-.947	1
47		5	max	40.86	1	90.911	3	-1.026	10	.011	3	.008	5	1.116	3
48			min	1.786	15	-63.156	1	-31.663	4	-.013	2	-.118	1	-.958	1
49		6	max	40.86	1	272.043	3	19.057	1	.011	3	-.005	12	.929	3
50			min	-7.851	5	-211.245	1	-25.856	5	-.013	2	-.116	1	-.817	1
51		7	max	40.86	1	453.175	3	53.61	1	.011	3	-.004	12	.556	3
52			min	-18.401	5	-359.335	1	-23.913	5	-.013	2	-.079	1	-.524	2
53		8	max	40.86	1	634.307	3	88.162	1	.011	3	0	10	-.001	15
54			min	-28.951	5	-507.425	1	-21.97	5	-.013	2	-.071	4	-.094	2
55		9	max	40.86	1	815.438	3	122.714	1	.011	3	.102	1	.519	1
56			min	-39.502	5	-655.515	1	-20.027	5	-.013	2	-.09	5	-.747	3
57		10	max	63.006	4	996.57	3	157.267	1	.013	2	.246	1	1.269	1
58			min	1.944	12	-803.605	1	-97.378	14	-.011	3	.005	12	-1.679	3
59		11	max	52.456	4	655.515	1	-3.665	12	.013	2	.124	4	.519	1
60			min	1.944	12	-815.438	3	-122.714	1	-.011	3	0	3	-.747	3
61		12	max	41.905	4	507.425	1	-2.409	12	.013	2	.068	5	-.001	15
62			min	1.944	12	-634.307	3	-88.162	1	-.011	3	-.006	1	-.094	2
63		13	max	40.86	1	359.335	1	-1.153	12	.013	2	.036	5	.556	3
64			min	1.944	12	-453.175	3	-53.61	1	-.011	3	-.079	1	-.524	2
65		14	max	40.86	1	211.245	1	.282	3	.013	2	.006	5	.929	3
66			min	1.944	12	-272.043	3	-32.351	4	-.011	3	-.116	1	-.817	1
67		15	max	40.86	1	63.156	1	15.495	1	.013	2	-.004	12	1.116	3
68			min	1.094	15	-90.911	3	-26.003	5	-.011	3	-.118	1	-.958	1
69		16	max	40.86	1	90.221	3	50.047	1	.013	2	-.002	12	1.116	3
70			min	-8.878	5	-84.942	2	-24.06	5	-.011	3	-.085	1	-.947	1
71		17	max	40.86	1	271.353	3	84.599	1	.013	2	.003	3	.93	3
72			min	-19.428	5	-233.024	1	-22.117	5	-.011	3	-.075	4	-.783	1
73		18	max	40.86	1	452.485	3	119.152	1	.013	2	.089	1	.558	3
74			min	-29.978	5	-381.114	1	-20.174	5	-.011	3	-.093	5	-.468	1
75		19	max	40.86	1	633.617	3	153.704	1	.013	2	.23	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-40.529	5	-529.204	1	-18.231	5	-.011	3	-.112	5	0	3
77	M15	1	max	76.408	5	721.594	2	-6.321	12	.013	2	.235	4	0	2
78			min	-42.754	1	-346.907	3	-153.694	1	-.009	3	.012	12	0	3
79		2	max	65.858	5	516.994	2	-5.065	12	.013	2	.16	4	.307	3
80			min	-42.754	1	-250.829	3	-119.142	1	-.009	3	.006	12	-.636	2
81		3	max	55.308	5	312.394	2	-3.81	12	.013	2	.095	5	.516	3
82			min	-42.754	1	-154.75	3	-84.589	1	-.009	3	-.015	1	-1.063	2
83		4	max	44.757	5	107.794	2	-2.554	12	.013	2	.054	5	.625	3
84			min	-42.754	1	-58.672	3	-50.564	4	-.009	3	-.085	1	-1.279	2
85		5	max	34.207	5	37.406	3	-1.055	10	.013	2	.014	5	.636	3
86			min	-42.754	1	-96.806	2	-41.422	4	-.009	3	-.118	1	-1.284	2
87		6	max	23.657	5	133.484	3	19.068	1	.013	2	-.005	12	.548	3
88			min	-42.754	1	-301.406	2	-35.6	5	-.009	3	-.117	1	-1.08	2
89		7	max	13.106	5	229.562	3	53.62	1	.013	2	-.004	12	.362	3
90			min	-42.754	1	-506.006	2	-33.658	5	-.009	3	-.079	1	-.665	2
91		8	max	2.556	5	325.64	3	88.172	1	.013	2	0	10	.076	3
92			min	-42.754	1	-710.605	2	-31.715	5	-.009	3	-.096	4	-.051	1
93		9	max	-2.374	12	421.718	3	122.724	1	.013	2	.102	1	.796	2
94			min	-42.754	1	-915.205	2	-29.772	5	-.009	3	-.124	5	-.308	3
95		10	max	-2.374	12	517.797	3	157.277	1	.009	3	.246	1	1.842	2
96			min	-42.754	1	-1119.805	2	-102.34	14	-.013	2	.006	12	-.79	3
97		11	max	2.876	5	915.205	2	-3.725	12	.009	3	.159	4	.796	2
98			min	-42.754	1	-421.718	3	-122.724	1	-.013	2	.001	12	-.308	3
99		12	max	-2.374	12	710.605	2	-2.469	12	.009	3	.092	5	.076	3
100			min	-42.754	1	-325.64	3	-88.172	1	-.013	2	-.006	1	-.051	1
101		13	max	-2.374	12	506.006	2	-1.214	12	.009	3	.05	5	.362	3
102			min	-42.754	1	-229.562	3	-53.62	1	-.013	2	-.079	1	-.665	2
103		14	max	-2.374	12	301.406	2	.182	3	.009	3	.011	5	.548	3
104			min	-42.754	1	-133.484	3	-42.128	4	-.013	2	-.117	1	-1.08	2
105		15	max	-2.374	12	96.806	2	15.485	1	.009	3	-.004	12	.636	3
106			min	-48.848	4	-37.406	3	-35.749	5	-.013	2	-.118	1	-1.284	2
107		16	max	-2.374	12	58.672	3	50.037	1	.009	3	-.002	12	.625	3
108			min	-59.399	4	-107.794	2	-33.806	5	-.013	2	-.085	1	-1.279	2
109		17	max	-2.374	12	154.75	3	84.589	1	.009	3	.002	3	.516	3
110			min	-69.949	4	-312.394	2	-31.863	5	-.013	2	-.101	4	-1.063	2
111		18	max	-2.374	12	250.829	3	119.142	1	.009	3	.089	1	.307	3
112			min	-80.499	4	-516.994	2	-29.92	5	-.013	2	-.128	5	-.636	2
113		19	max	-2.374	12	346.907	3	153.694	1	.009	3	.229	1	0	2
114			min	-91.05	4	-721.594	2	-27.977	5	-.013	2	-.158	5	0	5
115	M16	1	max	75.223	5	684.713	2	-6.004	12	.012	1	.201	1	0	2
116			min	-89.083	1	-318.094	3	-149.043	1	-.012	3	.01	12	0	3
117		2	max	64.673	5	480.113	2	-4.748	12	.012	1	.118	4	.278	3
118			min	-89.083	1	-222.016	3	-114.491	1	-.012	3	.004	12	-.599	2
119		3	max	54.123	5	275.514	2	-3.492	12	.012	1	.07	5	.456	3
120			min	-89.083	1	-125.937	3	-79.939	1	-.012	3	-.035	1	-.987	2
121		4	max	43.572	5	70.914	2	-2.236	12	.012	1	.039	5	.536	3
122			min	-89.083	1	-29.859	3	-45.386	1	-.012	3	-.099	1	-1.165	2
123		5	max	33.022	5	66.219	3	-.674	10	.012	1	.011	5	.518	3
124			min	-89.083	1	-133.686	2	-29.592	4	-.012	3	-.128	1	-1.133	2
125		6	max	22.472	5	162.297	3	23.718	1	.012	1	-.005	12	.4	3
126			min	-89.083	1	-338.286	2	-24.91	5	-.012	3	-.121	1	-.89	2
127		7	max	11.921	5	258.375	3	58.271	1	.012	1	-.004	12	.184	3
128			min	-89.083	1	-542.886	2	-22.967	5	-.012	3	-.079	1	-.437	2
129		8	max	1.371	5	354.453	3	92.823	1	.012	1	.001	2	.226	2
130			min	-89.083	1	-747.486	2	-21.024	5	-.012	3	-.065	4	-.131	3
131		9	max	-4.431	12	450.531	3	127.375	1	.012	1	.112	1	1.099	2
132			min	-89.083	1	-952.086	2	-19.081	5	-.012	3	-.084	5	-.544	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-4.431	12	546.61	3	161.927	1	.012	3	.26	1	2.183	2
134		min	-89.083	1	-1156.685	2	-99.414	14	-.012	1	.007	12	-1.057	3
135	11	max	-.424	15	952.086	2	-4.043	12	.012	3	.12	4	1.099	2
136		min	-89.083	1	-450.531	3	-127.375	1	-.012	1	.002	12	-.544	3
137	12	max	-4.431	12	747.486	2	-2.787	12	.012	3	.063	4	.226	2
138		min	-89.083	1	-354.453	3	-92.823	1	-.012	1	-.003	3	-.131	3
139	13	max	-4.431	12	542.886	2	-1.531	12	.012	3	.031	5	.184	3
140		min	-89.083	1	-258.375	3	-58.271	1	-.012	1	-.079	1	-.437	2
141	14	max	-4.431	12	338.286	2	-.275	12	.012	3	.002	5	.4	3
142		min	-89.083	1	-162.297	3	-32.805	4	-.012	1	-.121	1	-.89	2
143	15	max	-4.431	12	133.686	2	10.834	1	.012	3	-.004	12	.518	3
144		min	-89.083	1	-66.219	3	-25.597	5	-.012	1	-.128	1	-1.133	2
145	16	max	-4.431	12	29.859	3	45.386	1	.012	3	-.003	12	.536	3
146		min	-89.083	1	-70.914	2	-23.654	5	-.012	1	-.099	1	-1.165	2
147	17	max	-4.431	12	125.937	3	79.939	1	.012	3	0	3	.456	3
148		min	-89.083	1	-275.514	2	-21.711	5	-.012	1	-.082	4	-.987	2
149	18	max	-4.431	12	222.016	3	114.491	1	.012	3	.065	1	.278	3
150		min	-94.216	4	-480.113	2	-19.768	5	-.012	1	-.095	5	-.599	2
151	19	max	-4.431	12	318.094	3	149.043	1	.012	3	.201	1	0	2
152		min	-104.766	4	-684.713	2	-17.825	5	-.012	1	-.114	5	0	5
153	M2	1	max	1102.926	1	2.074	4	.783	1	0	0	3	0	1
154		min	-1443.063	3	.508	15	-53.364	4	0	4	0	1	0	1
155	2	max	1103.306	1	2.04	4	.783	1	0	3	0	1	0	15
156		min	-1442.778	3	.5	15	-53.694	4	0	4	-.014	4	0	4
157	3	max	1103.685	1	2.007	4	.783	1	0	3	0	1	0	15
158		min	-1442.494	3	.492	15	-54.023	4	0	4	-.028	4	-.001	4
159	4	max	1104.064	1	1.974	4	.783	1	0	3	0	1	0	15
160		min	-1442.209	3	.485	15	-54.352	4	0	4	-.041	4	-.002	4
161	5	max	1104.443	1	1.94	4	.783	1	0	3	0	1	0	15
162		min	-1441.925	3	.477	15	-54.682	4	0	4	-.055	4	-.002	4
163	6	max	1104.823	1	1.907	4	.783	1	0	3	0	1	0	15
164		min	-1441.641	3	.469	15	-55.011	4	0	4	-.069	4	-.003	4
165	7	max	1105.202	1	1.873	4	.783	1	0	3	.001	1	0	15
166		min	-1441.356	3	.461	15	-55.341	4	0	4	-.084	4	-.003	4
167	8	max	1105.581	1	1.84	4	.783	1	0	3	.001	1	0	15
168		min	-1441.072	3	.45	12	-55.67	4	0	4	-.098	4	-.004	4
169	9	max	1105.961	1	1.807	4	.783	1	0	3	.002	1	0	15
170		min	-1440.787	3	.437	12	-56	4	0	4	-.112	4	-.004	4
171	10	max	1106.34	1	1.773	4	.783	1	0	3	.002	1	-.001	15
172		min	-1440.503	3	.424	12	-56.329	4	0	4	-.126	4	-.004	4
173	11	max	1106.719	1	1.74	4	.783	1	0	3	.002	1	-.001	15
174		min	-1440.218	3	.411	12	-56.659	4	0	4	-.141	4	-.005	4
175	12	max	1107.098	1	1.706	4	.783	1	0	3	.002	1	-.001	15
176		min	-1439.934	3	.398	12	-56.988	4	0	4	-.156	4	-.005	4
177	13	max	1107.478	1	1.673	4	.783	1	0	3	.002	1	-.001	15
178		min	-1439.649	3	.385	12	-57.318	4	0	4	-.17	4	-.006	4
179	14	max	1107.857	1	1.64	4	.783	1	0	3	.003	1	-.002	12
180		min	-1439.365	3	.372	12	-57.647	4	0	4	-.185	4	-.006	4
181	15	max	1108.236	1	1.606	4	.783	1	0	3	.003	1	-.002	12
182		min	-1439.081	3	.359	12	-57.977	4	0	4	-.2	4	-.007	4
183	16	max	1108.615	1	1.573	4	.783	1	0	3	.003	1	-.002	12
184		min	-1438.796	3	.346	12	-58.306	4	0	4	-.215	4	-.007	4
185	17	max	1108.995	1	1.54	4	.783	1	0	3	.003	1	-.002	12
186		min	-1438.512	3	.333	12	-58.635	4	0	4	-.23	4	-.007	4
187	18	max	1109.374	1	1.506	4	.783	1	0	3	.003	1	-.002	12
188		min	-1438.227	3	.32	12	-58.965	4	0	4	-.245	4	-.008	4
189	19	max	1109.753	1	1.473	4	.783	1	0	3	.004	1	-.002	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1437.943	3	.307	12	-59.294	4	0	4	-.26	4	-.008	4
191	M3	1	max	544.65	2	8.011	4	1.181	4	0	3	0	.008	4
192		min	-680.637	3	1.896	15	.004	12	0	4	-.018	4	.002	12
193		2	max	544.48	2	7.241	4	1.722	4	0	3	0	.005	2
194		min	-680.765	3	1.715	15	.004	12	0	4	-.018	4	0	12
195		3	max	544.309	2	6.471	4	2.262	4	0	3	0	.003	2
196		min	-680.893	3	1.534	15	.004	12	0	4	-.017	4	0	3
197		4	max	544.139	2	5.701	4	2.803	4	0	3	0	0	2
198		min	-681.021	3	1.353	15	.004	12	0	4	-.016	4	-.002	3
199		5	max	543.969	2	4.931	4	3.343	4	0	3	0	0	15
200		min	-681.148	3	1.172	15	.004	12	0	4	-.015	4	-.003	3
201		6	max	543.798	2	4.161	4	3.884	4	0	3	0	1	15
202		min	-681.276	3	.991	15	.004	12	0	4	-.013	4	-.005	6
203		7	max	543.628	2	3.391	4	4.424	4	0	3	0	1	15
204		min	-681.404	3	.81	15	.004	12	0	4	-.011	5	-.006	6
205		8	max	543.457	2	2.621	4	4.965	4	0	3	0	1	15
206		min	-681.532	3	.629	15	.004	12	0	4	-.009	5	-.007	6
207		9	max	543.287	2	1.851	4	5.506	4	0	3	0	1	15
208		min	-681.659	3	.448	15	.004	12	0	4	-.007	5	-.008	6
209		10	max	543.117	2	1.081	4	6.046	4	0	3	0	1	15
210		min	-681.787	3	.258	12	.004	12	0	4	-.005	5	-.009	6
211		11	max	542.946	2	.406	2	6.587	4	0	3	0	1	15
212		min	-681.915	3	-.088	3	.004	12	0	4	-.002	5	-.009	6
213		12	max	542.776	2	-.095	15	7.127	4	0	3	0	4	15
214		min	-682.043	3	-.538	3	.004	12	0	4	0	12	-.009	6
215		13	max	542.606	2	-.276	15	7.668	4	0	3	.004	4	15
216		min	-682.17	3	-1.23	6	.004	12	0	4	0	12	-.009	6
217		14	max	542.435	2	-.457	15	8.208	4	0	3	.007	4	15
218		min	-682.298	3	-2	6	.004	12	0	4	0	12	-.008	6
219		15	max	542.265	2	-.638	15	8.749	4	0	3	.011	4	15
220		min	-682.426	3	-2.77	6	.004	12	0	4	0	12	-.007	6
221		16	max	542.095	2	-.819	15	9.289	4	0	3	.015	4	15
222		min	-682.554	3	-3.54	6	.004	12	0	4	0	12	-.006	6
223		17	max	541.924	2	-1	15	9.83	4	0	3	.019	4	15
224		min	-682.682	3	-4.31	6	.004	12	0	4	0	12	-.004	6
225		18	max	541.754	2	-1.181	15	10.37	4	0	3	.023	4	15
226		min	-682.809	3	-5.08	6	.004	12	0	4	0	12	-.002	6
227		19	max	541.584	2	-1.362	15	10.911	4	0	3	.027	4	1
228		min	-682.937	3	-5.85	6	.004	12	0	4	0	12	0	1
229	M4	1	max	1216.73	1	0	1	-.406	12	0	1	.017	4	1
230		min	-312.697	3	0	1	-241.758	4	0	1	0	12	0	1
231		2	max	1216.9	1	0	1	-.406	12	0	1	0	12	1
232		min	-312.569	3	0	1	-241.906	4	0	1	-.011	4	0	1
233		3	max	1217.07	1	0	1	-.406	12	0	1	0	12	1
234		min	-312.441	3	0	1	-242.053	4	0	1	-.039	4	0	1
235		4	max	1217.241	1	0	1	-.406	12	0	1	0	12	1
236		min	-312.314	3	0	1	-242.201	4	0	1	-.066	4	0	1
237		5	max	1217.411	1	0	1	-.406	12	0	1	0	12	1
238		min	-312.186	3	0	1	-242.348	4	0	1	-.094	4	0	1
239		6	max	1217.581	1	0	1	-.406	12	0	1	0	12	1
240		min	-312.058	3	0	1	-242.496	4	0	1	-.122	4	0	1
241		7	max	1217.752	1	0	1	-.406	12	0	1	0	12	1
242		min	-311.93	3	0	1	-242.644	4	0	1	-.15	4	0	1
243		8	max	1217.922	1	0	1	-.406	12	0	1	0	12	1
244		min	-311.803	3	0	1	-242.791	4	0	1	-.178	4	0	1
245		9	max	1218.092	1	0	1	-.406	12	0	1	0	12	1
246		min	-311.675	3	0	1	-242.939	4	0	1	-.206	4	0	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	1218.263	1	0	1	-406	12	0	1	0	12	0	1
248			min	-311.547	3	0	1	-243.087	4	0	1	-.234	4	0	1
249		11	max	1218.433	1	0	1	-406	12	0	1	0	12	0	1
250			min	-311.419	3	0	1	-243.234	4	0	1	-.261	4	0	1
251		12	max	1218.604	1	0	1	-406	12	0	1	0	12	0	1
252			min	-311.292	3	0	1	-243.382	4	0	1	-.289	4	0	1
253		13	max	1218.774	1	0	1	-406	12	0	1	0	12	0	1
254			min	-311.164	3	0	1	-243.53	4	0	1	-.317	4	0	1
255		14	max	1218.944	1	0	1	-406	12	0	1	0	12	0	1
256			min	-311.036	3	0	1	-243.677	4	0	1	-.345	4	0	1
257		15	max	1219.115	1	0	1	-406	12	0	1	0	12	0	1
258			min	-310.908	3	0	1	-243.825	4	0	1	-.373	4	0	1
259		16	max	1219.285	1	0	1	-406	12	0	1	0	12	0	1
260			min	-310.78	3	0	1	-243.972	4	0	1	-.401	4	0	1
261		17	max	1219.455	1	0	1	-406	12	0	1	0	12	0	1
262			min	-310.653	3	0	1	-244.12	4	0	1	-.429	4	0	1
263		18	max	1219.626	1	0	1	-406	12	0	1	0	12	0	1
264			min	-310.525	3	0	1	-244.268	4	0	1	-.457	4	0	1
265		19	max	1219.796	1	0	1	-406	12	0	1	0	12	0	1
266			min	-310.397	3	0	1	-244.415	4	0	1	-.485	4	0	1
267	M6	1	max	3525.2	1	2.658	2	0	1	0	1	0	4	0	1
268			min	-4692.816	3	-.142	3	-53.867	4	0	4	0	1	0	1
269		2	max	3525.579	1	2.632	2	0	1	0	1	0	1	0	3
270			min	-4692.532	3	-.161	3	-54.197	4	0	4	-.014	4	0	2
271		3	max	3525.958	1	2.606	2	0	1	0	1	0	1	0	3
272			min	-4692.247	3	-.181	3	-54.526	4	0	4	-.028	4	-.001	2
273		4	max	3526.337	1	2.58	2	0	1	0	1	0	1	0	3
274			min	-4691.963	3	-.2	3	-54.856	4	0	4	-.042	4	-.002	2
275		5	max	3526.717	1	2.554	2	0	1	0	1	0	1	0	3
276			min	-4691.678	3	-.22	3	-55.185	4	0	4	-.056	4	-.003	2
277		6	max	3527.096	1	2.528	2	0	1	0	1	0	1	0	3
278			min	-4691.394	3	-.239	3	-55.515	4	0	4	-.07	4	-.003	2
279		7	max	3527.475	1	2.502	2	0	1	0	1	0	1	0	3
280			min	-4691.109	3	-.259	3	-55.844	4	0	4	-.084	4	-.004	2
281		8	max	3527.854	1	2.476	2	0	1	0	1	0	1	0	3
282			min	-4690.825	3	-.278	3	-56.174	4	0	4	-.099	4	-.005	2
283		9	max	3528.234	1	2.45	2	0	1	0	1	0	1	0	3
284			min	-4690.54	3	-.298	3	-56.503	4	0	4	-.113	4	-.005	2
285		10	max	3528.613	1	2.424	2	0	1	0	1	0	1	0	3
286			min	-4690.256	3	-.318	3	-56.832	4	0	4	-.128	4	-.006	2
287		11	max	3528.992	1	2.398	2	0	1	0	1	0	1	0	3
288			min	-4689.972	3	-.337	3	-57.162	4	0	4	-.142	4	-.006	2
289		12	max	3529.371	1	2.372	2	0	1	0	1	0	1	0	3
290			min	-4689.687	3	-.357	3	-57.491	4	0	4	-.157	4	-.007	2
291		13	max	3529.751	1	2.346	2	0	1	0	1	0	1	0	3
292			min	-4689.403	3	-.376	3	-57.821	4	0	4	-.172	4	-.008	2
293		14	max	3530.13	1	2.319	2	0	1	0	1	0	1	0	3
294			min	-4689.118	3	-.396	3	-58.15	4	0	4	-.187	4	-.008	2
295		15	max	3530.509	1	2.293	2	0	1	0	1	0	1	0	3
296			min	-4688.834	3	-.415	3	-58.48	4	0	4	-.202	4	-.009	2
297		16	max	3530.888	1	2.267	2	0	1	0	1	0	1	.001	3
298			min	-4688.549	3	-.435	3	-58.809	4	0	4	-.217	4	-.009	2
299		17	max	3531.268	1	2.241	2	0	1	0	1	0	1	.001	3
300			min	-4688.265	3	-.454	3	-59.139	4	0	4	-.232	4	-.01	2
301		18	max	3531.647	1	2.215	2	0	1	0	1	0	1	.001	3
302			min	-4687.98	3	-.474	3	-59.468	4	0	4	-.247	4	-.011	2
303		19	max	3532.026	1	2.189	2	0	1	0	1	0	1	.001	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4687.696	3	-.493	3	-59.798	4	0	4	-.262	4	-.011	2
305	M7	1	max	2006.961	2	8.017	6	1.048	4	0	1	0	1	.011	2
306			min	-2136.488	3	1.882	15	0	1	0	4	-.019	4	-.001	3
307		2	max	2006.791	2	7.247	6	1.588	4	0	1	0	1	.008	2
308			min	-2136.616	3	1.701	15	0	1	0	4	-.018	4	-.003	3
309		3	max	2006.621	2	6.477	6	2.129	4	0	1	0	1	.006	2
310			min	-2136.744	3	1.52	15	0	1	0	4	-.017	4	-.004	3
311		4	max	2006.45	2	5.707	6	2.669	4	0	1	0	1	.004	2
312			min	-2136.872	3	1.339	15	0	1	0	4	-.016	4	-.005	3
313		5	max	2006.28	2	4.937	6	3.21	4	0	1	0	1	.002	2
314			min	-2136.999	3	1.158	15	0	1	0	4	-.015	4	-.006	3
315		6	max	2006.11	2	4.167	6	3.75	4	0	1	0	1	0	2
316			min	-2137.127	3	.977	15	0	1	0	4	-.014	4	-.007	3
317		7	max	2005.939	2	3.397	6	4.291	4	0	1	0	1	-.001	15
318			min	-2137.255	3	.796	15	0	1	0	4	-.012	4	-.008	3
319		8	max	2005.769	2	2.677	2	4.832	4	0	1	0	1	-.002	15
320			min	-2137.383	3	.525	12	0	1	0	4	-.01	4	-.008	3
321		9	max	2005.599	2	2.077	2	5.372	4	0	1	0	1	-.002	15
322			min	-2137.51	3	.225	12	0	1	0	4	-.008	4	-.008	3
323		10	max	2005.428	2	1.477	2	5.913	4	0	1	0	1	-.002	15
324			min	-2137.638	3	-.194	3	0	1	0	4	-.005	4	-.009	4
325		11	max	2005.258	2	.877	2	6.453	4	0	1	0	1	-.002	15
326			min	-2137.766	3	-.644	3	0	1	0	4	-.003	5	-.009	4
327		12	max	2005.088	2	.277	2	6.994	4	0	1	0	14	-.002	15
328			min	-2137.894	3	-1.094	3	0	1	0	4	0	5	-.009	4
329		13	max	2004.917	2	-.29	15	7.534	4	0	1	.003	4	-.002	15
330			min	-2138.021	3	-1.544	3	0	1	0	4	0	1	-.009	4
331		14	max	2004.747	2	-.471	15	8.075	4	0	1	.006	4	-.002	15
332			min	-2138.149	3	-1.994	3	0	1	0	4	0	1	-.008	4
333		15	max	2004.577	2	-.652	15	8.615	4	0	1	.01	4	-.002	15
334			min	-2138.277	3	-2.763	4	0	1	0	4	0	1	-.007	4
335		16	max	2004.406	2	-.833	15	9.156	4	0	1	.014	4	-.001	15
336			min	-2138.405	3	-3.533	4	0	1	0	4	0	1	-.006	4
337		17	max	2004.236	2	-1.014	15	9.697	4	0	1	.017	4	-.001	15
338			min	-2138.532	3	-4.303	4	0	1	0	4	0	1	-.004	4
339		18	max	2004.066	2	-1.195	15	10.237	4	0	1	.022	4	0	15
340			min	-2138.66	3	-5.073	4	0	1	0	4	0	1	-.002	4
341		19	max	2003.895	2	-1.376	15	10.778	4	0	1	.026	4	0	1
342			min	-2138.788	3	-5.843	4	0	1	0	4	0	1	0	1
343	M8	1	max	3472.919	1	0	1	0	1	0	1	.016	4	0	1
344			min	-1077.683	3	0	1	-235.466	4	0	1	0	1	0	1
345		2	max	3473.09	1	0	1	0	1	0	1	0	1	0	1
346			min	-1077.556	3	0	1	-235.613	4	0	1	-.011	4	0	1
347		3	max	3473.26	1	0	1	0	1	0	1	0	1	0	1
348			min	-1077.428	3	0	1	-235.761	4	0	1	-.038	4	0	1
349		4	max	3473.43	1	0	1	0	1	0	1	0	1	0	1
350			min	-1077.3	3	0	1	-235.908	4	0	1	-.065	4	0	1
351		5	max	3473.601	1	0	1	0	1	0	1	0	1	0	1
352			min	-1077.172	3	0	1	-236.056	4	0	1	-.092	4	0	1
353		6	max	3473.771	1	0	1	0	1	0	1	0	1	0	1
354			min	-1077.044	3	0	1	-236.204	4	0	1	-.119	4	0	1
355		7	max	3473.941	1	0	1	0	1	0	1	0	1	0	1
356			min	-1076.917	3	0	1	-236.351	4	0	1	-.146	4	0	1
357		8	max	3474.112	1	0	1	0	1	0	1	0	1	0	1
358			min	-1076.789	3	0	1	-236.499	4	0	1	-.173	4	0	1
359		9	max	3474.282	1	0	1	0	1	0	1	0	1	0	1
360			min	-1076.661	3	0	1	-236.647	4	0	1	-.201	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	3474.452	1	0	1	0	1	0	1	0	1	0	1
362			min	-1076.533	3	0	1	-236.794	4	0	1	-.228	4	0	1
363		11	max	3474.623	1	0	1	0	1	0	1	0	1	0	1
364			min	-1076.406	3	0	1	-236.942	4	0	1	-.255	4	0	1
365		12	max	3474.793	1	0	1	0	1	0	1	0	1	0	1
366			min	-1076.278	3	0	1	-237.09	4	0	1	-.282	4	0	1
367		13	max	3474.963	1	0	1	0	1	0	1	0	1	0	1
368			min	-1076.15	3	0	1	-237.237	4	0	1	-.309	4	0	1
369		14	max	3475.134	1	0	1	0	1	0	1	0	1	0	1
370			min	-1076.022	3	0	1	-237.385	4	0	1	-.337	4	0	1
371		15	max	3475.304	1	0	1	0	1	0	1	0	1	0	1
372			min	-1075.895	3	0	1	-237.532	4	0	1	-.364	4	0	1
373		16	max	3475.474	1	0	1	0	1	0	1	0	1	0	1
374			min	-1075.767	3	0	1	-237.68	4	0	1	-.391	4	0	1
375		17	max	3475.645	1	0	1	0	1	0	1	0	1	0	1
376			min	-1075.639	3	0	1	-237.828	4	0	1	-.419	4	0	1
377		18	max	3475.815	1	0	1	0	1	0	1	0	1	0	1
378			min	-1075.511	3	0	1	-237.975	4	0	1	-.446	4	0	1
379		19	max	3475.985	1	0	1	0	1	0	1	0	1	0	1
380			min	-1075.384	3	0	1	-238.123	4	0	1	-.473	4	0	1
381	M10	1	max	1102.926	1	1.983	6	-.038	12	0	1	0	1	0	1
382			min	-1443.063	3	.447	15	-53.782	4	0	5	0	3	0	1
383		2	max	1103.306	1	1.949	6	-.038	12	0	1	0	10	0	15
384			min	-1442.778	3	.439	15	-54.112	4	0	5	-.014	4	0	6
385		3	max	1103.685	1	1.916	6	-.038	12	0	1	0	10	0	15
386			min	-1442.494	3	.431	15	-54.441	4	0	5	-.028	4	0	6
387		4	max	1104.064	1	1.883	6	-.038	12	0	1	0	12	0	15
388			min	-1442.209	3	.423	15	-54.77	4	0	5	-.042	4	-.001	6
389		5	max	1104.443	1	1.849	6	-.038	12	0	1	0	12	0	15
390			min	-1441.925	3	.415	15	-55.1	4	0	5	-.056	4	-.002	6
391		6	max	1104.823	1	1.816	6	-.038	12	0	1	0	12	0	15
392			min	-1441.641	3	.408	15	-55.429	4	0	5	-.07	4	-.002	6
393		7	max	1105.202	1	1.782	6	-.038	12	0	1	0	12	0	15
394			min	-1441.356	3	.4	15	-55.759	4	0	5	-.084	4	-.003	6
395		8	max	1105.581	1	1.749	6	-.038	12	0	1	0	12	0	15
396			min	-1441.072	3	.392	15	-56.088	4	0	5	-.099	4	-.003	6
397		9	max	1105.961	1	1.716	6	-.038	12	0	1	0	12	0	15
398			min	-1440.787	3	.384	15	-56.418	4	0	5	-.113	4	-.004	6
399		10	max	1106.34	1	1.682	6	-.038	12	0	1	0	12	0	15
400			min	-1440.503	3	.376	15	-56.747	4	0	5	-.127	4	-.004	6
401		11	max	1106.719	1	1.649	6	-.038	12	0	1	0	12	-.001	15
402			min	-1440.218	3	.368	15	-57.077	4	0	5	-.142	4	-.005	6
403		12	max	1107.098	1	1.615	6	-.038	12	0	1	0	12	-.001	15
404			min	-1439.934	3	.361	15	-57.406	4	0	5	-.157	4	-.005	6
405		13	max	1107.478	1	1.582	6	-.038	12	0	1	0	12	-.001	15
406			min	-1439.649	3	.353	15	-57.736	4	0	5	-.171	4	-.005	6
407		14	max	1107.857	1	1.549	6	-.038	12	0	1	0	12	-.001	15
408			min	-1439.365	3	.345	15	-58.065	4	0	5	-.186	4	-.006	6
409		15	max	1108.236	1	1.521	2	-.038	12	0	1	0	12	-.001	15
410			min	-1439.081	3	.337	15	-58.395	4	0	5	-.201	4	-.006	6
411		16	max	1108.615	1	1.495	2	-.038	12	0	1	0	12	-.001	15
412			min	-1438.796	3	.329	15	-58.724	4	0	5	-.216	4	-.007	6
413		17	max	1108.995	1	1.469	2	-.038	12	0	1	0	12	-.002	15
414			min	-1438.512	3	.321	15	-59.053	4	0	5	-.231	4	-.007	6
415		18	max	1109.374	1	1.443	2	-.038	12	0	1	0	12	-.002	15
416			min	-1438.227	3	.313	15	-59.383	4	0	5	-.246	4	-.007	6
417		19	max	1109.753	1	1.417	2	-.038	12	0	1	0	12	-.002	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1437.943	3	.306	15	-59.712	4	0	5	-.262	4	-.008	6
419	M11	1	max	544.65	2	7.955	6	1.143	4	0	1	0	12	.008	6
420			min	-680.637	3	1.858	15	-.073	1	0	4	-.019	4	.002	15
421		2	max	544.48	2	7.185	6	1.683	4	0	1	0	12	.005	2
422			min	-680.765	3	1.677	15	-.073	1	0	4	-.018	4	0	12
423		3	max	544.309	2	6.415	6	2.224	4	0	1	0	12	.003	2
424			min	-680.893	3	1.496	15	-.073	1	0	4	-.017	4	0	3
425		4	max	544.139	2	5.645	6	2.764	4	0	1	0	12	0	2
426			min	-681.021	3	1.315	15	-.073	1	0	4	-.016	4	-.002	3
427		5	max	543.969	2	4.875	6	3.305	4	0	1	0	12	0	15
428			min	-681.148	3	1.134	15	-.073	1	0	4	-.015	4	-.003	3
429		6	max	543.798	2	4.105	6	3.845	4	0	1	0	12	-.001	15
430			min	-681.276	3	.953	15	-.073	1	0	4	-.013	4	-.005	4
431		7	max	543.628	2	3.335	6	4.386	4	0	1	0	12	-.002	15
432			min	-681.404	3	.772	15	-.073	1	0	4	-.012	4	-.006	4
433		8	max	543.457	2	2.565	6	4.926	4	0	1	0	12	-.002	15
434			min	-681.532	3	.591	15	-.073	1	0	4	-.01	4	-.008	4
435		9	max	543.287	2	1.795	6	5.467	4	0	1	0	12	-.002	15
436			min	-681.659	3	.41	15	-.073	1	0	4	-.008	4	-.009	4
437		10	max	543.117	2	1.025	6	6.008	4	0	1	0	12	-.002	15
438			min	-681.787	3	.229	15	-.073	1	0	4	-.005	4	-.009	4
439		11	max	542.946	2	.406	2	6.548	4	0	1	0	12	-.002	15
440			min	-681.915	3	-.088	3	-.073	1	0	4	-.002	4	-.01	4
441		12	max	542.776	2	-.133	15	7.089	4	0	1	0	5	-.002	15
442			min	-682.043	3	-.538	3	-.073	1	0	4	0	1	-.009	4
443		13	max	542.606	2	-.314	15	7.629	4	0	1	.004	5	-.002	15
444			min	-682.17	3	-1.286	4	-.073	1	0	4	0	1	-.009	4
445		14	max	542.435	2	-.495	15	8.17	4	0	1	.007	4	-.002	15
446			min	-682.298	3	-2.056	4	-.073	1	0	4	0	1	-.008	4
447		15	max	542.265	2	-.676	15	8.71	4	0	1	.01	4	-.002	15
448			min	-682.426	3	-2.826	4	-.073	1	0	4	0	1	-.007	4
449		16	max	542.095	2	-.857	15	9.251	4	0	1	.014	4	-.001	15
450			min	-682.554	3	-3.596	4	-.073	1	0	4	0	1	-.006	4
451		17	max	541.924	2	-1.038	15	9.791	4	0	1	.018	4	-.001	15
452			min	-682.682	3	-4.366	4	-.073	1	0	4	0	1	-.004	4
453		18	max	541.754	2	-1.219	15	10.332	4	0	1	.022	4	0	15
454			min	-682.809	3	-5.136	4	-.073	1	0	4	0	1	-.002	4
455		19	max	541.584	2	-1.4	15	10.873	4	0	1	.027	4	0	1
456			min	-682.937	3	-5.906	4	-.073	1	0	4	0	1	0	1
457	M12	1	max	1216.73	1	0	1	8.275	1	0	1	.017	4	0	1
458			min	-312.697	3	0	1	-237.332	4	0	1	0	1	0	1
459		2	max	1216.9	1	0	1	8.275	1	0	1	0	1	0	1
460			min	-312.569	3	0	1	-237.479	4	0	1	-.011	4	0	1
461		3	max	1217.07	1	0	1	8.275	1	0	1	.001	1	0	1
462			min	-312.441	3	0	1	-237.627	4	0	1	-.038	4	0	1
463		4	max	1217.241	1	0	1	8.275	1	0	1	.002	1	0	1
464			min	-312.314	3	0	1	-237.775	4	0	1	-.065	4	0	1
465		5	max	1217.411	1	0	1	8.275	1	0	1	.003	1	0	1
466			min	-312.186	3	0	1	-237.922	4	0	1	-.092	4	0	1
467		6	max	1217.581	1	0	1	8.275	1	0	1	.004	1	0	1
468			min	-312.058	3	0	1	-238.07	4	0	1	-.12	4	0	1
469		7	max	1217.752	1	0	1	8.275	1	0	1	.005	1	0	1
470			min	-311.93	3	0	1	-238.218	4	0	1	-.147	4	0	1
471		8	max	1217.922	1	0	1	8.275	1	0	1	.006	1	0	1
472			min	-311.803	3	0	1	-238.365	4	0	1	-.174	4	0	1
473		9	max	1218.092	1	0	1	8.275	1	0	1	.007	1	0	1
474			min	-311.675	3	0	1	-238.513	4	0	1	-.202	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475		10	max	1218.263	1	0	1	8.275	1	0	1	.008	1	0	1
476			min	-311.547	3	0	1	-238.661	4	0	1	-.229	4	0	1
477		11	max	1218.433	1	0	1	8.275	1	0	1	.009	1	0	1
478			min	-311.419	3	0	1	-238.808	4	0	1	-.257	4	0	1
479		12	max	1218.604	1	0	1	8.275	1	0	1	.01	1	0	1
480			min	-311.292	3	0	1	-238.956	4	0	1	-.284	4	0	1
481		13	max	1218.774	1	0	1	8.275	1	0	1	.011	1	0	1
482			min	-311.164	3	0	1	-239.103	4	0	1	-.312	4	0	1
483		14	max	1218.944	1	0	1	8.275	1	0	1	.012	1	0	1
484			min	-311.036	3	0	1	-239.251	4	0	1	-.339	4	0	1
485		15	max	1219.115	1	0	1	8.275	1	0	1	.013	1	0	1
486			min	-310.908	3	0	1	-239.399	4	0	1	-.366	4	0	1
487		16	max	1219.285	1	0	1	8.275	1	0	1	.014	1	0	1
488			min	-310.78	3	0	1	-239.546	4	0	1	-.394	4	0	1
489		17	max	1219.455	1	0	1	8.275	1	0	1	.015	1	0	1
490			min	-310.653	3	0	1	-239.694	4	0	1	-.421	4	0	1
491		18	max	1219.626	1	0	1	8.275	1	0	1	.016	1	0	1
492			min	-310.525	3	0	1	-239.842	4	0	1	-.449	4	0	1
493		19	max	1219.796	1	0	1	8.275	1	0	1	.017	1	0	1
494			min	-310.397	3	0	1	-239.989	4	0	1	-.477	4	0	1
495	M1	1	max	148.795	1	802.847	3	48.202	5	0	1	.199	1	0	3
496			min	-9.186	5	-491.813	1	-83.656	1	0	3	-.073	5	-.016	2
497		2	max	149.285	1	801.838	3	49.443	5	0	1	.155	1	.246	1
498			min	-8.958	5	-493.159	1	-83.656	1	0	3	-.047	5	-.423	3
499		3	max	410.244	3	573.134	2	-1	15	0	3	.111	1	.494	1
500			min	-243.61	2	-588.561	3	-83.057	1	0	1	-.022	5	-.829	3
501		4	max	410.612	3	571.788	2	-.164	15	0	3	.067	1	.197	1
502			min	-243.12	2	-589.571	3	-83.057	1	0	1	-.022	5	-.518	3
503		5	max	410.979	3	570.442	2	.895	5	0	3	.023	1	-.004	15
504			min	-242.63	2	-590.58	3	-83.057	1	0	1	-.022	5	-.207	3
505		6	max	411.347	3	569.096	2	2.136	5	0	3	-.001	12	.105	3
506			min	-242.14	2	-591.59	3	-83.057	1	0	1	-.025	4	-.427	2
507		7	max	411.714	3	567.749	2	3.378	5	0	3	-.003	12	.417	3
508			min	-241.65	2	-592.599	3	-83.057	1	0	1	-.064	1	-.727	2
509		8	max	412.081	3	566.403	2	4.619	5	0	3	-.006	12	.73	3
510			min	-241.16	2	-593.609	3	-83.057	1	0	1	-.108	1	-1.026	2
511		9	max	422.149	3	51.385	2	46.319	5	0	9	.065	1	.852	3
512			min	-180.677	2	.406	15	-123.947	1	0	3	-.114	5	-1.174	2
513		10	max	422.516	3	50.038	2	47.561	5	0	9	0	10	.831	3
514			min	-180.188	2	0	5	-123.947	1	0	3	-.09	4	-1.201	2
515		11	max	422.884	3	48.692	2	48.802	5	0	9	-.003	12	.809	3
516			min	-179.698	2	-1.686	4	-123.947	1	0	3	-.078	4	-1.227	2
517		12	max	432.851	3	389.045	3	131.41	5	0	2	.107	1	.706	3
518			min	-119.179	2	-673.015	2	-81.224	1	0	3	-.179	5	-1.088	2
519		13	max	433.219	3	388.035	3	132.651	5	0	2	.064	1	.501	3
520			min	-118.689	2	-674.361	2	-81.224	1	0	3	-.11	5	-.732	2
521		14	max	433.586	3	387.026	3	133.893	5	0	2	.021	1	.296	3
522			min	-118.199	2	-675.707	2	-81.224	1	0	3	-.039	5	-.376	2
523		15	max	433.954	3	386.016	3	135.134	5	0	2	.032	5	.093	3
524			min	-117.709	2	-677.053	2	-81.224	1	0	3	-.022	1	-.044	1
525		16	max	434.321	3	385.007	3	136.376	5	0	2	.103	5	.338	2
526			min	-117.219	2	-678.399	2	-81.224	1	0	3	-.065	1	-.111	3
527		17	max	434.689	3	383.997	3	137.617	5	0	2	.176	5	.697	2
528			min	-116.729	2	-679.745	2	-81.224	1	0	3	-.107	1	-.314	3
529		18	max	17.596	5	686.558	2	-4.431	12	0	5	.16	5	.351	2
530			min	-149.529	1	-317.144	3	-106.055	4	0	2	-.154	1	-.155	3
531		19	max	17.825	5	685.212	2	-4.431	12	0	5	.114	5	.012	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-149.039	1	-318.153	3	-104.813	4	0	2	-.201	1	-.012	1
533	M5	max	324.351	1	2675.195	3	84.061	5	0	1	0	1	.031	2
534		min	10.193	12	-1671.825	1	0	1	0	4	-.159	4	0	3
535		max	324.841	1	2674.185	3	85.302	5	0	1	0	1	.911	1
536		min	10.438	12	-1673.171	1	0	1	0	4	-.115	4	-1.412	3
537		max	1312.53	3	1712.018	2	36.927	4	0	4	0	1	1.753	1
538		min	-839.731	2	-1863.006	3	0	1	0	1	-.07	4	-2.768	3
539		max	1312.897	3	1710.672	2	38.169	4	0	4	0	1	.863	1
540		min	-839.241	2	-1864.015	3	0	1	0	1	-.051	4	-1.785	3
541		max	1313.265	3	1709.326	2	39.41	4	0	4	0	1	.015	9
542		min	-838.751	2	-1865.025	3	0	1	0	1	-.03	4	-.801	3
543		max	1313.632	3	1707.98	2	40.651	4	0	4	0	1	.183	3
544		min	-838.261	2	-1866.034	3	0	1	0	1	-.009	5	-.995	2
545		max	1314	3	1706.634	2	41.893	4	0	4	.013	4	1.168	3
546		min	-837.771	2	-1867.044	3	0	1	0	1	0	1	-1.895	2
547		max	1314.367	3	1705.288	2	43.134	4	0	4	.035	4	2.154	3
548		min	-837.281	2	-1868.053	3	0	1	0	1	0	1	-2.796	2
549		max	1329.242	3	172.259	2	149.051	4	0	1	0	1	2.479	3
550		min	-710.897	2	.406	15	0	1	0	1	-.16	4	-3.184	2
551		max	1329.609	3	170.913	2	150.293	4	0	1	0	1	2.4	3
552		min	-710.407	2	0	15	0	1	0	1	-.081	5	-3.275	2
553		max	1329.977	3	169.567	2	151.534	4	0	1	0	14	2.322	3
554		min	-709.917	2	-1.557	6	0	1	0	1	-.003	5	-3.364	2
555		max	1345.05	3	1211.135	3	184.163	4	0	1	0	1	2.038	3
556		min	-583.606	2	-2053.863	2	0	1	0	4	-.255	4	-3.012	2
557		max	1345.417	3	1210.125	3	185.405	4	0	1	0	1	1.4	3
558		min	-583.116	2	-2055.209	2	0	1	0	4	-.158	4	-1.928	2
559		max	1345.785	3	1209.115	3	186.646	4	0	1	0	1	.761	3
560		min	-582.626	2	-2056.555	2	0	1	0	4	-.06	4	-.843	2
561		max	1346.152	3	1208.106	3	187.887	4	0	1	.039	4	.242	2
562		min	-582.136	2	-2057.901	2	0	1	0	4	0	1	-.003	13
563		max	1346.52	3	1207.096	3	189.129	4	0	1	.139	4	1.329	2
564		min	-581.646	2	-2059.247	2	0	1	0	4	0	1	-.514	3
565		max	1346.887	3	1206.087	3	190.37	4	0	1	.239	4	2.416	2
566		min	-581.156	2	-2060.593	2	0	1	0	4	0	1	-1.15	3
567		max	-10.842	12	2317.392	2	0	1	0	4	.252	4	1.245	2
568		min	-324.352	1	-1092.518	3	-30.688	5	0	1	0	1	-.602	3
569		max	-10.597	12	2316.046	2	0	1	0	4	.237	4	.024	1
570		min	-323.862	1	-1093.527	3	-29.447	5	0	1	0	1	-.025	3
571	M9	max	148.795	1	802.847	3	83.656	1	0	3	-.011	12	0	3
572		min	6.206	12	-491.813	1	4.498	12	0	4	-.199	1	-.016	2
573		max	149.285	1	801.838	3	83.656	1	0	3	-.008	12	.246	1
574		min	6.451	12	-493.159	1	4.498	12	0	4	-.155	1	-.423	3
575		max	410.244	3	573.134	2	83.057	1	0	1	-.006	12	.494	1
576		min	-243.61	2	-588.561	3	4.457	12	0	3	-.111	1	-.829	3
577		max	410.612	3	571.788	2	83.057	1	0	1	-.004	12	.197	1
578		min	-243.12	2	-589.571	3	4.457	12	0	3	-.067	1	-.518	3
579		max	410.979	3	570.442	2	83.057	1	0	1	-.001	12	-.004	15
580		min	-242.63	2	-590.58	3	4.457	12	0	3	-.03	4	-.207	3
581		max	411.347	3	569.096	2	83.057	1	0	1	.02	1	.105	3
582		min	-242.14	2	-591.59	3	4.457	12	0	3	-.019	5	-.427	2
583		max	411.714	3	567.749	2	83.057	1	0	1	.064	1	.417	3
584		min	-241.65	2	-592.599	3	4.457	12	0	3	-.012	5	-.727	2
585		max	412.081	3	566.403	2	83.057	1	0	1	.108	1	.73	3
586		min	-241.16	2	-593.609	3	4.457	12	0	3	-.005	5	-1.026	2
587		max	422.149	3	51.385	2	123.947	1	0	3	-.003	12	.852	3
588		min	-180.677	2	.412	15	6.399	12	0	9	-.136	4	-1.174	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	422.516	3	50.038	2	123.947	1	0	3	0	1	.831	3
590		min	-180.188	2	.006	15	6.399	12	0	9	-.09	4	-1.201	2
591	11	max	422.884	3	48.692	2	123.947	1	0	3	.066	1	.809	3
592		min	-179.698	2	-1.639	6	6.399	12	0	9	-.057	5	-1.227	2
593	12	max	432.851	3	389.045	3	159.422	4	0	3	-.005	12	.706	3
594		min	-119.179	2	-673.015	2	4.028	12	0	2	-.216	4	-1.088	2
595	13	max	433.219	3	388.035	3	160.663	4	0	3	-.003	12	.501	3
596		min	-118.689	2	-674.361	2	4.028	12	0	2	-.131	4	-.732	2
597	14	max	433.586	3	387.026	3	161.905	4	0	3	-.001	12	.296	3
598		min	-118.199	2	-675.707	2	4.028	12	0	2	-.046	4	-.376	2
599	15	max	433.954	3	386.016	3	163.146	4	0	3	.039	4	.093	3
600		min	-117.709	2	-677.053	2	4.028	12	0	2	.001	12	-.044	1
601	16	max	434.321	3	385.007	3	164.388	4	0	3	.126	4	.338	2
602		min	-117.219	2	-678.399	2	4.028	12	0	2	.003	12	-.111	3
603	17	max	434.689	3	383.997	3	165.629	4	0	3	.213	4	.697	2
604		min	-116.729	2	-679.745	2	4.028	12	0	2	.005	12	-.314	3
605	18	max	-6.249	12	686.558	2	89.166	1	0	2	.212	4	.351	2
606		min	-149.529	1	-317.144	3	-76.578	5	0	3	.008	12	-.155	3
607	19	max	-6.004	12	685.212	2	89.166	1	0	2	.201	1	.012	3
608		min	-149.039	1	-318.153	3	-75.337	5	0	3	.01	12	-.012	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.125	2	.007	3	1.018e-2	2	NC	1	NC	1
2			min	-.511	4	-.025	3	-.004	2	-2.07e-3	3	NC	1	NC	1
3		2	max	0	1	.275	3	.028	1	1.16e-2	2	NC	5	NC	2
4			min	-.511	4	-.048	1	-.014	5	-2.095e-3	3	738.879	3	8220.151	1
5		3	max	0	1	.518	3	.066	1	1.301e-2	2	NC	5	NC	3
6			min	-.511	4	-.176	1	-.017	5	-2.12e-3	3	408.318	3	3398.79	1
7		4	max	0	1	.666	3	.099	1	1.443e-2	2	NC	5	NC	3
8			min	-.511	4	-.247	1	-.012	5	-2.145e-3	3	321.143	3	2261.92	1
9		5	max	0	1	.7	3	.115	1	1.584e-2	2	NC	5	NC	3
10			min	-.511	4	-.248	1	-.003	5	-2.169e-3	3	306.092	3	1935.633	1
11		6	max	0	1	.623	3	.111	1	1.726e-2	2	NC	5	NC	3
12			min	-.511	4	-.182	1	.004	15	-2.194e-3	3	342.48	3	2015.829	1
13		7	max	0	1	.458	3	.086	1	1.867e-2	2	NC	5	NC	3
14			min	-.511	4	-.064	1	.001	10	-2.219e-3	3	459.494	3	2592.932	1
15		8	max	0	1	.248	3	.049	1	2.009e-2	2	NC	2	NC	2
16			min	-.511	4	.002	15	-.003	10	-2.244e-3	3	811.472	3	4596.429	1
17		9	max	0	1	.229	2	.023	3	2.15e-2	2	NC	4	NC	1
18			min	-.511	4	.005	15	-.008	2	-2.269e-3	3	2148.082	2	NC	1
19		10	max	0	1	.283	2	.023	3	2.292e-2	2	NC	3	NC	1
20			min	-.511	4	-.028	3	-.015	2	-2.293e-3	3	1403.509	2	NC	1
21		11	max	0	12	.229	2	.023	3	2.15e-2	2	NC	4	NC	1
22			min	-.511	4	.005	15	-.011	5	-2.269e-3	3	2148.082	2	NC	1
23		12	max	0	12	.248	3	.049	1	2.009e-2	2	NC	2	NC	2
24			min	-.511	4	.002	15	-.011	5	-2.244e-3	3	811.472	3	4596.429	1
25		13	max	0	12	.458	3	.086	1	1.867e-2	2	NC	5	NC	3
26			min	-.511	4	-.064	1	-.004	5	-2.219e-3	3	459.494	3	2592.932	1
27		14	max	0	12	.623	3	.111	1	1.726e-2	2	NC	5	NC	3
28			min	-.511	4	-.182	1	.004	15	-2.194e-3	3	342.48	3	2015.829	1
29		15	max	0	12	.7	3	.115	1	1.584e-2	2	NC	5	NC	3
30			min	-.511	4	-.248	1	.006	10	-2.169e-3	3	306.092	3	1935.633	1
31		16	max	0	12	.666	3	.099	1	1.443e-2	2	NC	5	NC	3
32			min	-.511	4	-.247	1	.005	10	-2.145e-3	3	321.143	3	2261.92	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.518	3	.066	1	1.301e-2	2	NC	5	NC	3
34		min	-.511	4	-.176	1	.003	10	-2.12e-3	3	408.318	3	3398.79	1
35	18	max	0	12	.275	3	.028	1	1.16e-2	2	NC	5	NC	2
36		min	-.511	4	-.048	1	0	10	-2.095e-3	3	738.879	3	8220.151	1
37	19	max	0	12	.125	2	.007	3	1.018e-2	2	NC	1	NC	1
38		min	-.511	4	-.025	3	-.004	2	-2.07e-3	3	NC	1	NC	1
39	M14	1	max	0	.254	3	.007	3	5.958e-3	2	NC	1	NC	1
40		min	-.397	4	-.392	2	-.003	2	-4.539e-3	3	NC	1	NC	1
41	2	max	0	1	.56	3	.019	1	7.099e-3	2	NC	5	NC	1
42		min	-.397	4	-.681	2	-.02	5	-5.49e-3	3	725.791	3	NC	1
43	3	max	0	1	.82	3	.053	1	8.241e-3	2	NC	5	NC	2
44		min	-.397	4	-.933	2	-.024	5	-6.442e-3	3	392.044	3	4293.04	1
45	4	max	0	1	1.004	3	.084	1	9.382e-3	2	NC	15	NC	3
46		min	-.397	4	-1.122	2	-.017	5	-7.393e-3	3	296.005	3	2666.906	1
47	5	max	0	1	1.095	3	.102	1	1.052e-2	2	NC	15	NC	3
48		min	-.397	4	-1.234	2	-.003	5	-8.345e-3	3	263.747	2	2197.497	1
49	6	max	0	1	1.095	3	.1	1	1.167e-2	2	NC	15	NC	3
50		min	-.397	4	-1.268	2	.004	10	-9.296e-3	3	253.449	2	2233.779	1
51	7	max	0	1	1.017	3	.08	1	1.281e-2	2	NC	15	NC	3
52		min	-.397	4	-1.235	2	.001	10	-1.025e-2	3	263.166	2	2823.413	1
53	8	max	0	1	.895	3	.046	1	1.395e-2	2	NC	15	NC	2
54		min	-.397	4	-1.16	2	-.003	10	-1.12e-2	3	288.929	2	4927.967	1
55	9	max	0	1	.774	3	.027	4	1.509e-2	2	NC	5	NC	1
56		min	-.397	4	-1.078	2	-.007	2	-1.215e-2	3	323.464	2	8212.436	4
57	10	max	0	1	.716	3	.021	3	1.623e-2	2	NC	5	NC	1
58		min	-.397	4	-1.038	2	-.014	2	-1.31e-2	3	343.698	2	NC	1
59	11	max	0	12	.774	3	.021	3	1.509e-2	2	NC	5	NC	1
60		min	-.397	4	-1.078	2	-.02	5	-1.215e-2	3	323.464	2	NC	1
61	12	max	0	12	.895	3	.046	1	1.395e-2	2	NC	15	NC	2
62		min	-.397	4	-1.16	2	-.023	5	-1.12e-2	3	288.929	2	4927.967	1
63	13	max	0	12	1.017	3	.08	1	1.281e-2	2	NC	15	NC	3
64		min	-.397	4	-1.235	2	-.015	5	-1.025e-2	3	263.166	2	2823.413	1
65	14	max	0	12	1.095	3	.1	1	1.167e-2	2	NC	15	NC	3
66		min	-.397	4	-1.268	2	0	5	-9.296e-3	3	253.449	2	2233.779	1
67	15	max	0	12	1.095	3	.102	1	1.052e-2	2	NC	15	NC	3
68		min	-.397	4	-1.234	2	.005	10	-8.345e-3	3	263.747	2	2197.497	1
69	16	max	0	12	1.004	3	.084	1	9.382e-3	2	NC	15	NC	3
70		min	-.397	4	-1.122	2	.004	10	-7.393e-3	3	296.005	3	2666.906	1
71	17	max	0	12	.82	3	.053	1	8.241e-3	2	NC	5	NC	2
72		min	-.397	4	-.933	2	.002	10	-6.442e-3	3	392.044	3	4293.04	1
73	18	max	0	12	.56	3	.028	4	7.099e-3	2	NC	5	NC	1
74		min	-.397	4	-.681	2	0	10	-5.49e-3	3	725.791	3	7977.959	4
75	19	max	0	12	.254	3	.007	3	5.958e-3	2	NC	1	NC	1
76		min	-.397	4	-.392	2	-.003	2	-4.539e-3	3	NC	1	NC	1
77	M15	1	max	0	.26	3	.006	3	3.854e-3	3	NC	1	NC	1
78		min	-.331	4	-.391	2	-.003	2	-6.172e-3	2	NC	1	NC	1
79	2	max	0	12	.459	3	.019	1	4.664e-3	3	NC	5	NC	1
80		min	-.331	4	-.757	2	-.029	5	-7.357e-3	2	606.617	2	7459.647	5
81	3	max	0	12	.634	3	.053	1	5.473e-3	3	NC	5	NC	2
82		min	-.331	4	-1.071	2	-.035	5	-8.542e-3	2	326.449	2	4279.025	1
83	4	max	0	12	.767	3	.084	1	6.283e-3	3	NC	15	NC	3
84		min	-.331	4	-1.298	2	-.025	5	-9.728e-3	2	244.895	2	2659.573	1
85	5	max	0	12	.847	3	.102	1	7.093e-3	3	NC	15	NC	3
86		min	-.331	4	-1.418	2	-.007	5	-1.091e-2	2	216.107	2	2191.518	1
87	6	max	0	12	.875	3	.101	1	7.903e-3	3	NC	15	NC	3
88		min	-.331	4	-1.433	2	.004	10	-1.21e-2	2	213.085	2	2226.846	1
89	7	max	0	12	.858	3	.08	1	8.712e-3	3	NC	15	NC	3





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.331	4	-1.359	2	.002	10	-1.328e-2	2	229.486	2	2811.517	1
91	8	max	0	12	.811	3	.05	4	9.522e-3	3	NC	15	NC	2
92		min	-.331	4	-1.23	2	-.003	10	-1.447e-2	2	264.706	2	4439.536	4
93	9	max	0	12	.758	3	.034	4	1.033e-2	3	NC	5	NC	1
94		min	-.331	4	-1.099	2	-.006	10	-1.565e-2	2	313.51	2	6459.74	4
95	10	max	0	1	.732	3	.019	3	1.114e-2	3	NC	5	NC	1
96		min	-.331	4	-1.037	2	-.013	2	-1.684e-2	2	343.788	2	NC	1
97	11	max	0	1	.758	3	.02	3	1.033e-2	3	NC	5	NC	1
98		min	-.331	4	-1.099	2	-.027	5	-1.565e-2	2	313.51	2	8068.386	5
99	12	max	0	1	.811	3	.046	1	9.522e-3	3	NC	15	NC	2
100		min	-.331	4	-1.23	2	-.032	5	-1.447e-2	2	264.706	2	4890.282	1
101	13	max	0	1	.858	3	.08	1	8.712e-3	3	NC	15	NC	3
102		min	-.331	4	-1.359	2	-.021	5	-1.328e-2	2	229.486	2	2811.517	1
103	14	max	0	1	.875	3	.101	1	7.903e-3	3	NC	15	NC	3
104		min	-.331	4	-1.433	2	-.002	5	-1.21e-2	2	213.085	2	2226.846	1
105	15	max	0	1	.847	3	.102	1	7.093e-3	3	NC	15	NC	3
106		min	-.331	4	-1.418	2	.006	10	-1.091e-2	2	216.107	2	2191.518	1
107	16	max	0	1	.767	3	.084	1	6.283e-3	3	NC	15	NC	3
108		min	-.331	4	-1.298	2	.005	10	-9.728e-3	2	244.895	2	2659.573	1
109	17	max	0	1	.634	3	.054	4	5.473e-3	3	NC	5	NC	2
110		min	-.331	4	-1.071	2	.002	10	-8.542e-3	2	326.449	2	4122	4
111	18	max	0	1	.459	3	.036	4	4.664e-3	3	NC	5	NC	1
112		min	-.331	4	-.757	2	0	10	-7.357e-3	2	606.617	2	6122.157	4
113	19	max	0	1	.26	3	.006	3	3.854e-3	3	NC	1	NC	1
114		min	-.331	4	-.391	2	-.003	2	-6.172e-3	2	NC	1	NC	1
115	M16	1	max	0	12	.11	.005	3	6.891e-3	3	NC	1	NC	1
116		min	-.137	4	-.087	3	-.003	2	-8.525e-3	2	NC	1	NC	1
117	2	max	0	12	.013	3	.028	1	8.021e-3	3	NC	5	NC	2
118		min	-.137	4	-.127	2	-.022	5	-9.556e-3	1	933.9	2	8262.846	1
119	3	max	0	12	.09	3	.066	1	9.151e-3	3	NC	5	NC	3
120		min	-.137	4	-.316	2	-.027	5	-1.063e-2	1	520.249	2	3403.928	1
121	4	max	0	12	.129	3	.099	1	1.028e-2	3	NC	5	NC	3
122		min	-.137	4	-.424	2	-.021	5	-1.171e-2	1	415.366	2	2260.099	1
123	5	max	0	12	.125	3	.116	1	1.141e-2	3	NC	5	NC	3
124		min	-.137	4	-.436	2	-.008	5	-1.279e-2	1	406.565	2	1929.75	1
125	6	max	0	12	.077	3	.112	1	1.254e-2	3	NC	5	NC	3
126		min	-.137	4	-.354	2	.004	15	-1.386e-2	1	478.312	2	2003.651	1
127	7	max	0	12	0	12	.087	1	1.367e-2	3	NC	5	NC	3
128		min	-.137	4	-.199	2	.003	10	-1.494e-2	1	718.258	2	2563.162	1
129	8	max	0	12	.026	1	.05	1	1.48e-2	3	NC	3	NC	2
130		min	-.137	4	-.098	3	-.002	10	-1.602e-2	1	1867.993	2	4477.671	1
131	9	max	0	12	.175	1	.024	4	1.593e-2	3	NC	4	NC	1
132		min	-.137	4	-.182	3	-.006	10	-1.709e-2	1	2342.48	3	9214.797	4
133	10	max	0	1	.242	1	.017	3	1.706e-2	3	NC	5	NC	1
134		min	-.137	4	-.219	3	-.012	2	-1.817e-2	1	1680.369	1	NC	1
135	11	max	0	1	.175	1	.017	3	1.593e-2	3	NC	4	NC	1
136		min	-.137	4	-.182	3	-.017	5	-1.709e-2	1	2342.48	3	NC	1
137	12	max	0	1	.026	1	.05	1	1.48e-2	3	NC	3	NC	2
138		min	-.137	4	-.098	3	-.018	5	-1.602e-2	1	1867.993	2	4477.671	1
139	13	max	0	1	0	12	.087	1	1.367e-2	3	NC	5	NC	3
140		min	-.137	4	-.199	2	-.009	5	-1.494e-2	1	718.258	2	2563.162	1
141	14	max	0	1	.077	3	.112	1	1.254e-2	3	NC	5	NC	3
142		min	-.137	4	-.354	2	.004	15	-1.386e-2	1	478.312	2	2003.651	1
143	15	max	0	1	.125	3	.116	1	1.141e-2	3	NC	5	NC	3
144		min	-.137	4	-.436	2	.007	10	-1.279e-2	1	406.565	2	1929.75	1
145	16	max	0	1	.129	3	.099	1	1.028e-2	3	NC	5	NC	3
146		min	-.137	4	-.424	2	.006	10	-1.171e-2	1	415.366	2	2260.099	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147		17	max	0	1	.09	3	.066	1	9.151e-3	3	NC	5	NC	3
148			min	-1.137	4	-.316	2	.004	10	-1.063e-2	1	520.249	2	3403.928	1
149		18	max	0	1	.013	3	.032	4	8.021e-3	3	NC	5	NC	2
150			min	-1.137	4	-.127	2	0	10	-9.556e-3	1	933.9	2	6961.031	4
151		19	max	0	1	.11	2	.005	3	6.891e-3	3	NC	1	NC	1
152			min	-1.137	4	-.087	3	-.003	2	-8.525e-3	2	NC	1	NC	1
153	M2	1	max	.006	1	.006	2	.006	1	1.296e-3	5	NC	1	NC	2
154			min	-.008	3	-.01	3	-.482	4	-1.695e-4	1	9721.297	2	114.818	4
155		2	max	.006	1	.005	2	.006	1	1.377e-3	5	NC	1	NC	2
156			min	-.007	3	-.009	3	-.443	4	-1.582e-4	1	NC	1	125.057	4
157		3	max	.005	1	.004	2	.005	1	1.458e-3	5	NC	1	NC	1
158			min	-.007	3	-.009	3	-.403	4	-1.469e-4	1	NC	1	137.222	4
159		4	max	.005	1	.004	2	.005	1	1.54e-3	5	NC	1	NC	1
160			min	-.006	3	-.009	3	-.365	4	-1.355e-4	1	NC	1	151.817	4
161		5	max	.005	1	.003	2	.004	1	1.621e-3	5	NC	1	NC	1
162			min	-.006	3	-.008	3	-.327	4	-1.242e-4	1	NC	1	169.525	4
163		6	max	.004	1	.002	2	.004	1	1.702e-3	5	NC	1	NC	1
164			min	-.006	3	-.008	3	-.289	4	-1.128e-4	1	NC	1	191.293	4
165		7	max	.004	1	.002	2	.003	1	1.783e-3	5	NC	1	NC	1
166			min	-.005	3	-.007	3	-.253	4	-1.015e-4	1	NC	1	218.467	4
167		8	max	.004	1	.001	2	.003	1	1.864e-3	5	NC	1	NC	1
168			min	-.005	3	-.007	3	-.219	4	-9.016e-5	1	NC	1	253.008	4
169		9	max	.003	1	0	2	.002	1	1.949e-3	4	NC	1	NC	1
170			min	-.004	3	-.006	3	-.186	4	-7.882e-5	1	NC	1	297.871	4
171		10	max	.003	1	0	2	.002	1	2.034e-3	4	NC	1	NC	1
172			min	-.004	3	-.006	3	-.155	4	-6.748e-5	1	NC	1	357.674	4
173		11	max	.003	1	0	2	.002	1	2.119e-3	4	NC	1	NC	1
174			min	-.003	3	-.005	3	-.126	4	-5.615e-5	1	NC	1	439.979	4
175		12	max	.002	1	0	15	.001	1	2.205e-3	4	NC	1	NC	1
176			min	-.003	3	-.005	3	-.099	4	-4.481e-5	1	NC	1	557.868	4
177		13	max	.002	1	0	15	0	1	2.29e-3	4	NC	1	NC	1
178			min	-.003	3	-.004	3	-.075	4	-3.347e-5	1	NC	1	735.64	4
179		14	max	.002	1	0	15	0	1	2.375e-3	4	NC	1	NC	1
180			min	-.002	3	-.004	3	-.054	4	-2.213e-5	1	NC	1	1022.771	4
181		15	max	.001	1	0	15	0	1	2.46e-3	4	NC	1	NC	1
182			min	-.002	3	-.003	3	-.036	4	-1.079e-5	1	NC	1	1533.444	4
183		16	max	0	1	0	15	0	1	2.545e-3	4	NC	1	NC	1
184			min	-.001	3	-.002	3	-.021	4	-6.05e-7	3	NC	1	2583.831	4
185		17	max	0	1	0	15	0	1	2.631e-3	4	NC	1	NC	1
186			min	0	3	-.002	3	-.01	4	2.599e-7	12	NC	1	5349.425	4
187		18	max	0	1	0	15	0	1	2.716e-3	4	NC	1	NC	1
188			min	0	3	0	3	-.003	4	8.82e-7	12	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.801e-3	4	NC	1	NC	1
190			min	0	1	0	1	0	1	1.504e-6	12	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-4.903e-7	12	NC	1	NC	1
192			min	0	1	0	1	0	1	-6.595e-4	4	NC	1	NC	1
193		2	max	0	3	0	15	.013	4	8.709e-6	1	NC	1	NC	1
194			min	0	2	-.002	6	0	12	-5.744e-5	5	NC	1	NC	1
195		3	max	0	3	0	15	.026	4	5.489e-4	4	NC	1	NC	1
196			min	0	2	-.003	6	0	12	1.384e-6	12	NC	1	NC	1
197		4	max	0	3	-.001	15	.038	4	1.153e-3	4	NC	1	NC	1
198			min	0	2	-.005	6	0	12	2.322e-6	12	NC	1	NC	1
199		5	max	.001	3	-.001	15	.049	4	1.757e-3	4	NC	1	NC	1
200			min	-.001	2	-.007	6	0	12	3.259e-6	12	NC	1	NC	1
201		6	max	.002	3	-.002	15	.059	4	2.361e-3	4	NC	1	NC	1
202			min	-.001	2	-.009	6	0	12	4.197e-6	12	NC	1	9619.519	5
203		7	max	.002	3	-.002	15	.069	4	2.966e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.01	6	0	12	5.134e-6	12	9096.319	6	9746.446	5
205		8	max	.002	3	-.002	15	.079	4	3.57e-3	4	NC	1	NC	1
206			min	-.002	2	-.011	6	0	12	6.071e-6	12	8127.051	6	NC	1
207		9	max	.003	3	-.003	15	.087	4	4.174e-3	4	NC	1	NC	1
208			min	-.002	2	-.012	6	0	12	7.009e-6	12	7549.595	6	NC	1
209		10	max	.003	3	-.003	15	.096	4	4.778e-3	4	NC	2	NC	1
210			min	-.002	2	-.013	6	0	12	7.946e-6	12	7262.042	6	NC	1
211		11	max	.003	3	-.003	15	.104	4	5.382e-3	4	NC	2	NC	1
212			min	-.003	2	-.013	6	0	12	8.884e-6	12	7221.553	6	NC	1
213		12	max	.004	3	-.003	15	.113	4	5.987e-3	4	NC	2	NC	1
214			min	-.003	2	-.012	6	0	12	9.821e-6	12	7427.85	6	NC	1
215		13	max	.004	3	-.003	15	.121	4	6.591e-3	4	NC	1	NC	1
216			min	-.003	2	-.012	6	0	12	1.076e-5	12	7925.168	6	NC	1
217		14	max	.004	3	-.002	15	.129	4	7.195e-3	4	NC	1	NC	1
218			min	-.003	2	-.011	6	0	12	1.17e-5	12	8825.617	6	NC	1
219		15	max	.005	3	-.002	15	.138	4	7.799e-3	4	NC	1	NC	1
220			min	-.004	2	-.009	6	0	12	1.263e-5	12	NC	1	NC	1
221		16	max	.005	3	-.001	15	.148	4	8.403e-3	4	NC	1	NC	1
222			min	-.004	2	-.007	1	0	12	1.357e-5	12	NC	1	NC	1
223		17	max	.005	3	0	15	.158	4	9.008e-3	4	NC	1	NC	1
224			min	-.004	2	-.006	1	0	12	1.451e-5	12	NC	1	NC	1
225		18	max	.006	3	0	15	.168	4	9.612e-3	4	NC	1	NC	1
226			min	-.004	2	-.004	1	0	12	1.545e-5	12	NC	1	NC	1
227		19	max	.006	3	0	5	.18	4	1.022e-2	4	NC	1	NC	1
228			min	-.005	2	-.003	1	0	12	1.638e-5	12	NC	1	NC	1
229	M4	1	max	.003	1	.004	2	0	12	2.022e-5	1	NC	1	NC	2
230			min	0	3	-.006	3	-.18	4	-4.257e-4	5	NC	1	137.7	4
231		2	max	.003	1	.004	2	0	12	2.022e-5	1	NC	1	NC	2
232			min	0	3	-.006	3	-.166	4	-4.257e-4	5	NC	1	149.815	4
233		3	max	.003	1	.004	2	0	12	2.022e-5	1	NC	1	NC	2
234			min	0	3	-.005	3	-.151	4	-4.257e-4	5	NC	1	164.228	4
235		4	max	.002	1	.003	2	0	12	2.022e-5	1	NC	1	NC	2
236			min	0	3	-.005	3	-.137	4	-4.257e-4	5	NC	1	181.537	4
237		5	max	.002	1	.003	2	0	12	2.022e-5	1	NC	1	NC	2
238			min	0	3	-.005	3	-.122	4	-4.257e-4	5	NC	1	202.554	4
239		6	max	.002	1	.003	2	0	12	2.022e-5	1	NC	1	NC	2
240			min	0	3	-.004	3	-.109	4	-4.257e-4	5	NC	1	228.405	4
241		7	max	.002	1	.003	2	0	12	2.022e-5	1	NC	1	NC	2
242			min	0	3	-.004	3	-.095	4	-4.257e-4	5	NC	1	260.691	4
243		8	max	.002	1	.003	2	0	12	2.022e-5	1	NC	1	NC	2
244			min	0	3	-.004	3	-.082	4	-4.257e-4	5	NC	1	301.742	4
245		9	max	.002	1	.002	2	0	12	2.022e-5	1	NC	1	NC	1
246			min	0	3	-.003	3	-.07	4	-4.257e-4	5	NC	1	355.07	4
247		10	max	.001	1	.002	2	0	12	2.022e-5	1	NC	1	NC	1
248			min	0	3	-.003	3	-.058	4	-4.257e-4	5	NC	1	426.164	4
249		11	max	.001	1	.002	2	0	12	2.022e-5	1	NC	1	NC	1
250			min	0	3	-.003	3	-.047	4	-4.257e-4	5	NC	1	524.002	4
251		12	max	.001	1	.002	2	0	12	2.022e-5	1	NC	1	NC	1
252			min	0	3	-.002	3	-.037	4	-4.257e-4	5	NC	1	664.119	4
253		13	max	0	1	.001	2	0	12	2.022e-5	1	NC	1	NC	1
254			min	0	3	-.002	3	-.028	4	-4.257e-4	5	NC	1	875.345	4
255		14	max	0	1	.001	2	0	12	2.022e-5	1	NC	1	NC	1
256			min	0	3	-.002	3	-.02	4	-4.257e-4	5	NC	1	1216.34	4
257		15	max	0	1	0	2	0	12	2.022e-5	1	NC	1	NC	1
258			min	0	3	-.001	3	-.014	4	-4.257e-4	5	NC	1	1822.327	4
259		16	max	0	1	0	2	0	12	2.022e-5	1	NC	1	NC	1
260			min	0	3	0	3	-.008	4	-4.257e-4	5	NC	1	3067.135	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	2.022e-5	1	NC	1	NC	1
262			min	0	3	0	3	-.004	4	-4.257e-4	5	NC	1	6336.926	4
263		18	max	0	1	0	2	0	12	2.022e-5	1	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-4.257e-4	5	NC	1	NC	1
265		19	max	0	1	0	1	0	1	2.022e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-4.257e-4	5	NC	1	NC	1
267	M6	1	max	.019	1	.021	2	0	1	1.358e-3	4	NC	4	NC	1
268			min	-.025	3	-.031	3	-.486	4	0	1	1783.778	3	113.821	4
269		2	max	.018	1	.02	2	0	1	1.437e-3	4	NC	4	NC	1
270			min	-.024	3	-.029	3	-.446	4	0	1	1892.14	3	123.973	4
271		3	max	.017	1	.018	2	0	1	1.517e-3	4	NC	4	NC	1
272			min	-.022	3	-.027	3	-.407	4	0	1	2014.472	3	136.034	4
273		4	max	.016	1	.016	2	0	1	1.596e-3	4	NC	4	NC	1
274			min	-.021	3	-.026	3	-.368	4	0	1	2153.598	3	150.505	4
275		5	max	.015	1	.014	2	0	1	1.676e-3	4	NC	4	NC	1
276			min	-.019	3	-.024	3	-.329	4	0	1	2313.153	3	168.063	4
277		6	max	.014	1	.013	2	0	1	1.756e-3	4	NC	4	NC	1
278			min	-.018	3	-.022	3	-.292	4	0	1	2497.89	3	189.647	4
279		7	max	.013	1	.011	2	0	1	1.835e-3	4	NC	1	NC	1
280			min	-.017	3	-.02	3	-.256	4	0	1	2714.15	3	216.592	4
281		8	max	.011	1	.01	2	0	1	1.915e-3	4	NC	1	NC	1
282			min	-.015	3	-.019	3	-.221	4	0	1	2970.581	3	250.843	4
283		9	max	.01	1	.008	2	0	1	1.995e-3	4	NC	1	NC	1
284			min	-.014	3	-.017	3	-.187	4	0	1	3279.292	3	295.331	4
285		10	max	.009	1	.007	2	0	1	2.074e-3	4	NC	1	NC	1
286			min	-.012	3	-.015	3	-.156	4	0	1	3657.778	3	354.639	4
287		11	max	.008	1	.006	2	0	1	2.154e-3	4	NC	1	NC	1
288			min	-.011	3	-.013	3	-.127	4	0	1	4132.286	3	436.265	4
289		12	max	.007	1	.004	2	0	1	2.234e-3	4	NC	1	NC	1
290			min	-.01	3	-.012	3	-.1	4	0	1	4744.068	3	553.191	4
291		13	max	.006	1	.003	2	0	1	2.313e-3	4	NC	1	NC	1
292			min	-.008	3	-.01	3	-.076	4	0	1	5561.881	3	729.527	4
293		14	max	.005	1	.002	2	0	1	2.393e-3	4	NC	1	NC	1
294			min	-.007	3	-.008	3	-.055	4	0	1	6709.5	3	1014.374	4
295		15	max	.004	1	.002	2	0	1	2.473e-3	4	NC	1	NC	1
296			min	-.006	3	-.007	3	-.036	4	0	1	8434.488	3	1521.071	4
297		16	max	.003	1	0	2	0	1	2.552e-3	4	NC	1	NC	1
298			min	-.004	3	-.005	3	-.022	4	0	1	NC	1	2563.55	4
299		17	max	.002	1	0	2	0	1	2.632e-3	4	NC	1	NC	1
300			min	-.003	3	-.003	3	-.01	4	0	1	NC	1	5309.549	4
301		18	max	.001	1	0	2	0	1	2.711e-3	4	NC	1	NC	1
302			min	-.001	3	-.002	3	-.003	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.791e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-6.558e-4	4	NC	1	NC	1
307		2	max	.001	3	0	2	.013	4	0	1	NC	1	NC	1
308			min	0	2	-.003	3	0	1	-6.501e-5	5	NC	1	NC	1
309		3	max	.002	3	0	2	.026	4	5.259e-4	4	NC	1	NC	1
310			min	-.002	2	-.005	3	0	1	0	1	NC	1	NC	1
311		4	max	.003	3	-.001	15	.038	4	1.117e-3	4	NC	1	NC	1
312			min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
313		5	max	.004	3	-.002	15	.049	4	1.708e-3	4	NC	1	NC	1
314			min	-.004	2	-.009	3	0	1	0	1	NC	1	9431.673	4
315		6	max	.005	3	-.002	15	.059	4	2.298e-3	4	NC	1	NC	1
316			min	-.005	2	-.011	3	0	1	0	1	8876.547	3	8894.172	4
317		7	max	.006	3	-.002	15	.069	4	2.889e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318			min	-.006	2	-.012	3	0	1	0	1	7919.026	3	8921.707	4
319		8	max	.007	3	-.003	15	.078	4	3.48e-3	4	NC	1	NC	1
320			min	-.007	2	-.013	3	0	1	0	1	7350.941	3	9449.568	4
321		9	max	.008	3	-.003	15	.087	4	4.071e-3	4	NC	1	NC	1
322			min	-.008	2	-.014	3	0	1	0	1	7055.256	3	NC	1
323		10	max	.009	3	-.003	15	.095	4	4.662e-3	4	NC	1	NC	1
324			min	-.009	2	-.014	3	0	1	0	1	6979.271	3	NC	1
325		11	max	.01	3	-.003	15	.103	4	5.253e-3	4	NC	1	NC	1
326			min	-.01	2	-.014	3	0	1	0	1	7109.351	3	NC	1
327		12	max	.011	3	-.003	15	.111	4	5.844e-3	4	NC	1	NC	1
328			min	-.011	2	-.014	3	0	1	0	1	7456.159	4	NC	1
329		13	max	.012	3	-.003	15	.119	4	6.434e-3	4	NC	1	NC	1
330			min	-.012	2	-.013	3	0	1	0	1	7954.092	4	NC	1
331		14	max	.013	3	-.003	15	.127	4	7.025e-3	4	NC	1	NC	1
332			min	-.013	2	-.012	3	0	1	0	1	8856.647	4	NC	1
333		15	max	.015	3	-.002	15	.136	4	7.616e-3	4	NC	1	NC	1
334			min	-.014	2	-.01	3	0	1	0	1	NC	1	NC	1
335		16	max	.016	3	-.002	15	.145	4	8.207e-3	4	NC	1	NC	1
336			min	-.015	2	-.009	1	0	1	0	1	NC	1	NC	1
337		17	max	.017	3	-.001	15	.154	4	8.798e-3	4	NC	1	NC	1
338			min	-.016	2	-.008	1	0	1	0	1	NC	1	NC	1
339		18	max	.018	3	0	15	.164	4	9.389e-3	4	NC	1	NC	1
340			min	-.017	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.019	3	0	15	.176	4	9.98e-3	4	NC	1	NC	1
342			min	-.018	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	1	.016	2	0	1	0	1	NC	1	NC	1
344			min	-.003	3	-.019	3	-.176	4	-4.732e-4	4	NC	1	141.234	4
345		2	max	.008	1	.015	2	0	1	0	1	NC	1	NC	1
346			min	-.002	3	-.018	3	-.161	4	-4.732e-4	4	NC	1	153.664	4
347		3	max	.007	1	.014	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.017	3	-.147	4	-4.732e-4	4	NC	1	168.451	4
349		4	max	.007	1	.013	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.016	3	-.133	4	-4.732e-4	4	NC	1	186.209	4
351		5	max	.006	1	.012	2	0	1	0	1	NC	1	NC	1
352			min	-.002	3	-.015	3	-.119	4	-4.732e-4	4	NC	1	207.772	4
353		6	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
354			min	-.002	3	-.013	3	-.106	4	-4.732e-4	4	NC	1	234.294	4
355		7	max	.006	1	.011	2	0	1	0	1	NC	1	NC	1
356			min	-.002	3	-.012	3	-.093	4	-4.732e-4	4	NC	1	267.417	4
357		8	max	.005	1	.01	2	0	1	0	1	NC	1	NC	1
358			min	-.002	3	-.011	3	-.08	4	-4.732e-4	4	NC	1	309.532	4
359		9	max	.005	1	.009	2	0	1	0	1	NC	1	NC	1
360			min	-.001	3	-.01	3	-.068	4	-4.732e-4	4	NC	1	364.243	4
361		10	max	.004	1	.008	2	0	1	0	1	NC	1	NC	1
362			min	-.001	3	-.009	3	-.057	4	-4.732e-4	4	NC	1	437.18	4
363		11	max	.004	1	.007	2	0	1	0	1	NC	1	NC	1
364			min	-.001	3	-.008	3	-.046	4	-4.732e-4	4	NC	1	537.556	4
365		12	max	.003	1	.006	2	0	1	0	1	NC	1	NC	1
366			min	0	3	-.007	3	-.036	4	-4.732e-4	4	NC	1	681.307	4
367		13	max	.003	1	.005	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.006	3	-.028	4	-4.732e-4	4	NC	1	898.013	4
369		14	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.005	3	-.02	4	-4.732e-4	4	NC	1	1247.855	4
371		15	max	.002	1	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	-.013	4	-4.732e-4	4	NC	1	1869.568	4
373		16	max	.001	1	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	-.008	4	-4.732e-4	4	NC	1	3146.693	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375		17	max	0	1	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	-.004	4	-4.732e-4	4	NC	1	6501.411	4
377		18	max	0	1	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	-.001	4	-4.732e-4	4	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	-4.732e-4	4	NC	1	NC	1
381	M10	1	max	.006	1	.006	2	0	12	1.358e-3	4	NC	1	NC	2
382			min	-.008	3	-.01	3	-.486	4	9.695e-6	12	9721.297	2	114	4
383		2	max	.006	1	.005	2	0	12	1.437e-3	4	NC	1	NC	2
384			min	-.007	3	-.009	3	-.446	4	9.073e-6	12	NC	1	124.167	4
385		3	max	.005	1	.004	2	0	12	1.516e-3	4	NC	1	NC	1
386			min	-.007	3	-.009	3	-.406	4	8.451e-6	12	NC	1	136.248	4
387		4	max	.005	1	.004	2	0	12	1.595e-3	4	NC	1	NC	1
388			min	-.006	3	-.009	3	-.367	4	7.829e-6	12	NC	1	150.741	4
389		5	max	.005	1	.003	2	0	12	1.674e-3	4	NC	1	NC	1
390			min	-.006	3	-.008	3	-.329	4	7.206e-6	12	NC	1	168.326	4
391		6	max	.004	1	.002	2	0	12	1.754e-3	4	NC	1	NC	1
392			min	-.006	3	-.008	3	-.291	4	6.584e-6	12	NC	1	189.945	4
393		7	max	.004	1	.002	2	0	12	1.833e-3	4	NC	1	NC	1
394			min	-.005	3	-.007	3	-.255	4	5.962e-6	12	NC	1	216.932	4
395		8	max	.004	1	.001	2	0	12	1.912e-3	4	NC	1	NC	1
396			min	-.005	3	-.007	3	-.22	4	5.34e-6	12	NC	1	251.238	4
397		9	max	.003	1	0	2	0	12	1.991e-3	4	NC	1	NC	1
398			min	-.004	3	-.006	3	-.187	4	4.718e-6	12	NC	1	295.796	4
399		10	max	.003	1	0	2	0	12	2.07e-3	4	NC	1	NC	1
400			min	-.004	3	-.006	3	-.156	4	4.095e-6	12	NC	1	355.198	4
401		11	max	.003	1	0	2	0	12	2.149e-3	4	NC	1	NC	1
402			min	-.003	3	-.005	3	-.127	4	3.473e-6	12	NC	1	436.954	4
403		12	max	.002	1	0	2	0	12	2.228e-3	4	NC	1	NC	1
404			min	-.003	3	-.005	3	-.1	4	2.851e-6	12	NC	1	554.068	4
405		13	max	.002	1	0	2	0	12	2.308e-3	4	NC	1	NC	1
406			min	-.003	3	-.004	3	-.076	4	2.229e-6	12	NC	1	730.689	4
407		14	max	.002	1	0	15	0	12	2.387e-3	4	NC	1	NC	1
408			min	-.002	3	-.004	3	-.054	4	1.607e-6	12	NC	1	1015.999	4
409		15	max	.001	1	0	15	0	12	2.466e-3	4	NC	1	NC	1
410			min	-.002	3	-.003	3	-.036	4	7.773e-7	10	NC	1	1523.532	4
411		16	max	0	1	0	15	0	12	2.545e-3	4	NC	1	NC	1
412			min	-.001	3	-.002	3	-.022	4	-5.505e-7	1	NC	1	2567.762	4
413		17	max	0	1	0	15	0	12	2.624e-3	4	NC	1	NC	1
414			min	0	3	-.002	3	-.01	4	-1.189e-5	1	NC	1	5318.524	4
415		18	max	0	1	0	15	0	12	2.703e-3	4	NC	1	NC	1
416			min	0	3	0	4	-.003	4	-2.323e-5	1	NC	1	NC	1
417		19	max	0	1	0	1	0	1	2.783e-3	4	NC	1	NC	1
418			min	0	1	0	1	0	1	-3.457e-5	1	NC	1	NC	1
419	M11	1	max	0	1	0	1	0	1	1.095e-5	1	NC	1	NC	1
420			min	0	1	0	1	0	1	-6.536e-4	4	NC	1	NC	1
421		2	max	0	3	0	15	.013	4	-4.471e-7	12	NC	1	NC	1
422			min	0	2	-.002	4	0	1	-5.991e-5	4	NC	1	NC	1
423		3	max	0	3	0	15	.026	4	5.338e-4	4	NC	1	NC	1
424			min	0	2	-.004	4	0	1	-2.837e-5	1	NC	1	NC	1
425		4	max	0	3	-.001	15	.038	4	1.127e-3	4	NC	1	NC	1
426			min	0	2	-.005	4	0	1	-4.803e-5	1	NC	1	NC	1
427		5	max	.001	3	-.002	15	.049	4	1.721e-3	4	NC	1	NC	1
428			min	-.001	2	-.007	4	0	1	-6.769e-5	1	NC	1	9846.136	4
429		6	max	.002	3	-.002	15	.059	4	2.315e-3	4	NC	1	NC	1
430			min	-.001	2	-.009	4	-.001	1	-8.735e-5	1	NC	1	9342.473	4
431		7	max	.002	3	-.003	15	.069	4	2.908e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.011	4	-.001	1	-1.07e-4	1	8763.561	4	9445.362	4
433		8	max	.002	3	-.003	15	.078	4	3.502e-3	4	NC	1	NC	1
434			min	-.002	2	-.012	4	-.002	1	-1.267e-4	1	7851.683	4	NC	1
435		9	max	.003	3	-.003	15	.087	4	4.096e-3	4	NC	1	NC	1
436			min	-.002	2	-.013	4	-.002	1	-1.463e-4	1	7310.672	4	NC	1
437		10	max	.003	3	-.003	15	.095	4	4.69e-3	4	NC	2	NC	1
438			min	-.002	2	-.013	4	-.002	1	-1.66e-4	1	7045.788	4	NC	1
439		11	max	.003	3	-.003	15	.103	4	5.283e-3	4	NC	2	NC	1
440			min	-.003	2	-.014	4	-.002	1	-1.856e-4	1	7017.836	4	NC	1
441		12	max	.004	3	-.003	15	.111	4	5.877e-3	4	NC	2	NC	1
442			min	-.003	2	-.013	4	-.003	1	-2.053e-4	1	7228.118	4	NC	1
443		13	max	.004	3	-.003	15	.119	4	6.471e-3	4	NC	1	NC	1
444			min	-.003	2	-.013	4	-.003	1	-2.25e-4	1	7720.832	4	NC	1
445		14	max	.004	3	-.003	15	.128	4	7.064e-3	4	NC	1	NC	1
446			min	-.003	2	-.011	4	-.004	1	-2.446e-4	1	8606.173	4	NC	1
447		15	max	.005	3	-.002	15	.136	4	7.658e-3	4	NC	1	NC	1
448			min	-.004	2	-.01	4	-.004	1	-2.643e-4	1	NC	1	NC	1
449		16	max	.005	3	-.002	15	.145	4	8.252e-3	4	NC	1	NC	1
450			min	-.004	2	-.008	4	-.005	1	-2.839e-4	1	NC	1	NC	1
451		17	max	.005	3	-.002	15	.155	4	8.845e-3	4	NC	1	NC	1
452			min	-.004	2	-.006	1	-.005	1	-3.036e-4	1	NC	1	NC	1
453		18	max	.006	3	0	15	.165	4	9.439e-3	4	NC	1	NC	1
454			min	-.004	2	-.004	1	-.006	1	-3.233e-4	1	NC	1	NC	1
455		19	max	.006	3	0	15	.177	4	1.003e-2	4	NC	1	NC	1
456			min	-.005	2	-.003	1	-.006	1	-3.429e-4	1	NC	1	NC	1
457	M12	1	max	.003	1	.004	2	.006	1	-1.242e-6	12	NC	1	NC	2
458			min	0	3	-.006	3	-.177	4	-4.357e-4	4	NC	1	140.278	4
459		2	max	.003	1	.004	2	.006	1	-1.242e-6	12	NC	1	NC	2
460			min	0	3	-.006	3	-.163	4	-4.357e-4	4	NC	1	152.619	4
461		3	max	.003	1	.004	2	.005	1	-1.242e-6	12	NC	1	NC	2
462			min	0	3	-.005	3	-.148	4	-4.357e-4	4	NC	1	167.301	4
463		4	max	.002	1	.003	2	.005	1	-1.242e-6	12	NC	1	NC	2
464			min	0	3	-.005	3	-.134	4	-4.357e-4	4	NC	1	184.933	4
465		5	max	.002	1	.003	2	.004	1	-1.242e-6	12	NC	1	NC	2
466			min	0	3	-.005	3	-.12	4	-4.357e-4	4	NC	1	206.342	4
467		6	max	.002	1	.003	2	.004	1	-1.242e-6	12	NC	1	NC	2
468			min	0	3	-.004	3	-.107	4	-4.357e-4	4	NC	1	232.676	4
469		7	max	.002	1	.003	2	.003	1	-1.242e-6	12	NC	1	NC	2
470			min	0	3	-.004	3	-.093	4	-4.357e-4	4	NC	1	265.564	4
471		8	max	.002	1	.003	2	.003	1	-1.242e-6	12	NC	1	NC	2
472			min	0	3	-.004	3	-.081	4	-4.357e-4	4	NC	1	307.381	4
473		9	max	.002	1	.002	2	.002	1	-1.242e-6	12	NC	1	NC	1
474			min	0	3	-.003	3	-.069	4	-4.357e-4	4	NC	1	361.704	4
475		10	max	.001	1	.002	2	.002	1	-1.242e-6	12	NC	1	NC	1
476			min	0	3	-.003	3	-.057	4	-4.357e-4	4	NC	1	434.124	4
477		11	max	.001	1	.002	2	.002	1	-1.242e-6	12	NC	1	NC	1
478			min	0	3	-.003	3	-.046	4	-4.357e-4	4	NC	1	533.788	4
479		12	max	.001	1	.002	2	.001	1	-1.242e-6	12	NC	1	NC	1
480			min	0	3	-.002	3	-.037	4	-4.357e-4	4	NC	1	676.519	4
481		13	max	0	1	.001	2	0	1	-1.242e-6	12	NC	1	NC	1
482			min	0	3	-.002	3	-.028	4	-4.357e-4	4	NC	1	891.685	4
483		14	max	0	1	.001	2	0	1	-1.242e-6	12	NC	1	NC	1
484			min	0	3	-.002	3	-.02	4	-4.357e-4	4	NC	1	1239.04	4
485		15	max	0	1	0	2	0	1	-1.242e-6	12	NC	1	NC	1
486			min	0	3	-.001	3	-.013	4	-4.357e-4	4	NC	1	1856.328	4
487		16	max	0	1	0	2	0	1	-1.242e-6	12	NC	1	NC	1
488			min	0	3	0	3	-.008	4	-4.357e-4	4	NC	1	3124.348	4



Company : Schletter, Inc.  
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Job Number :  
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Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489		17	max	0	1	0	2	0	1	-1.242e-6	12	NC	1	NC	1
490			min	0	3	0	3	-.004	4	-4.357e-4	4	NC	1	6455.101	4
491		18	max	0	1	0	2	0	1	-1.242e-6	12	NC	1	NC	1
492			min	0	3	0	3	-.001	4	-4.357e-4	4	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-1.242e-6	12	NC	1	NC	1
494			min	0	1	0	1	0	1	-4.357e-4	4	NC	1	NC	1
495	M1	1	max	.007	3	.125	2	.511	4	1.383e-2	1	NC	1	NC	1
496			min	-.004	2	-.025	3	0	12	-2.528e-2	3	NC	1	NC	1
497		2	max	.007	3	.061	2	.497	4	7.594e-3	4	NC	4	NC	1
498			min	-.004	2	-.011	3	-.005	1	-1.251e-2	3	1783.195	2	NC	1
499		3	max	.007	3	.011	3	.482	4	1.265e-2	4	NC	5	NC	1
500			min	-.004	2	-.009	2	-.006	1	-1.263e-4	3	859.632	2	8273.89	5
501		4	max	.007	3	.049	3	.467	4	1.104e-2	4	NC	5	NC	1
502			min	-.004	2	-.087	2	-.006	1	-4.83e-3	3	542.84	2	5802.695	5
503		5	max	.007	3	.097	3	.452	4	9.428e-3	4	NC	5	NC	1
504			min	-.004	2	-.169	2	-.004	1	-9.535e-3	3	391.889	2	4552.81	5
505		6	max	.007	3	.149	3	.437	4	1.336e-2	2	NC	15	NC	1
506			min	-.004	2	-.248	2	-.002	1	-1.424e-2	3	308.719	2	3804.775	5
507		7	max	.007	3	.199	3	.421	4	1.781e-2	2	NC	15	NC	1
508			min	-.004	2	-.319	2	0	3	-1.894e-2	3	259.618	2	3300.267	4
509		8	max	.007	3	.241	3	.404	4	2.226e-2	2	9271.976	15	NC	1
510			min	-.003	2	-.375	2	0	12	-2.365e-2	3	230.572	2	2943.103	4
511		9	max	.007	3	.268	3	.386	4	2.524e-2	2	8669.185	15	NC	1
512			min	-.003	2	-.41	2	0	1	-2.385e-2	3	215.451	2	2726.091	4
513		10	max	.006	3	.278	3	.367	4	2.726e-2	2	8485.456	15	NC	1
514			min	-.003	2	-.422	2	0	12	-2.106e-2	3	211.013	2	2662.262	4
515		11	max	.006	3	.271	3	.345	4	2.928e-2	2	8668.924	15	NC	1
516			min	-.003	2	-.41	2	0	12	-1.827e-2	3	216.163	2	2721.724	4
517		12	max	.006	3	.248	3	.321	4	2.826e-2	2	9271.375	15	NC	1
518			min	-.003	2	-.373	2	0	1	-1.538e-2	3	232.733	2	2918.869	4
519		13	max	.006	3	.211	3	.295	4	2.266e-2	2	NC	15	NC	1
520			min	-.003	2	-.315	2	0	1	-1.231e-2	3	264.867	2	3429.154	4
521		14	max	.006	3	.164	3	.266	4	1.706e-2	2	NC	15	NC	1
522			min	-.003	2	-.242	2	0	12	-9.243e-3	3	319.912	2	4497.809	4
523		15	max	.006	3	.111	3	.236	4	1.146e-2	2	NC	5	NC	1
524			min	-.003	2	-.161	2	0	12	-6.177e-3	3	414.88	2	6828.013	4
525		16	max	.006	3	.057	3	.208	4	8.633e-3	4	NC	5	NC	1
526			min	-.003	2	-.08	2	0	12	-3.11e-3	3	591.211	2	NC	1
527		17	max	.005	3	.004	3	.181	4	9.696e-3	4	NC	5	NC	1
528			min	-.003	2	-.006	2	0	12	-4.41e-5	3	969.077	2	NC	1
529		18	max	.005	3	.056	2	.157	4	9.982e-3	2	NC	4	NC	1
530			min	-.003	2	-.043	3	0	12	-4.162e-3	3	2061.473	2	NC	1
531		19	max	.005	3	.11	2	.137	4	2.006e-2	2	NC	1	NC	1
532			min	-.003	2	-.087	3	0	1	-8.447e-3	3	NC	1	NC	1
533	M5	1	max	.023	3	.283	2	.511	4	0	1	NC	1	NC	1
534			min	-.015	2	-.028	3	0	1	-3.747e-6	4	NC	1	NC	1
535		2	max	.023	3	.137	2	.5	4	6.48e-3	4	NC	5	NC	1
536			min	-.015	2	-.011	3	0	1	0	1	789.861	2	NC	1
537		3	max	.023	3	.034	3	.486	4	1.276e-2	4	NC	5	NC	1
538			min	-.015	2	-.028	2	0	1	0	1	371.879	2	6821.232	4
539		4	max	.022	3	.131	3	.471	4	1.04e-2	4	NC	15	NC	1
540			min	-.015	2	-.224	2	0	1	0	1	227.831	2	5116.505	4
541		5	max	.022	3	.264	3	.454	4	8.034e-3	4	7339.462	15	NC	1
542			min	-.015	2	-.437	2	0	1	0	1	160.479	2	4259.476	4
543		6	max	.022	3	.412	3	.438	4	5.67e-3	4	5647.666	15	NC	1
544			min	-.015	2	-.648	2	0	1	0	1	124.113	2	3725.199	4
545		7	max	.021	3	.557	3	.42	4	3.306e-3	4	4671.262	15	NC	1





Company : Schletter, Inc.  
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Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546			min	-.014	2	-.838	2	0	1	0	1	103	2	3328.524	4
547		8	max	.021	3	.679	3	.404	4	9.413e-4	4	4103.799	15	NC	1
548			min	-.014	2	-.991	2	0	1	0	1	90.678	2	2986.422	4
549		9	max	.02	3	.757	3	.387	4	0	1	3812.892	15	NC	1
550			min	-.014	2	-1.088	2	0	1	-2.39e-6	5	84.342	2	2723.291	4
551		10	max	.02	3	.785	3	.366	4	0	1	3725.259	15	NC	1
552			min	-.014	2	-1.121	2	0	1	-2.29e-6	5	82.487	2	2681.403	4
553		11	max	.019	3	.766	3	.345	4	0	1	3812.995	15	NC	1
554			min	-.013	2	-1.089	2	0	1	-2.19e-6	5	84.632	2	2751.915	4
555		12	max	.019	3	.7	3	.322	4	6.946e-4	4	4104.041	15	NC	1
556			min	-.013	2	-.988	2	0	1	0	1	91.627	2	2870.84	4
557		13	max	.018	3	.592	3	.295	4	2.44e-3	4	4671.751	15	NC	1
558			min	-.013	2	-.828	2	0	1	0	1	105.463	2	3369.594	4
559		14	max	.018	3	.457	3	.265	4	4.185e-3	4	5648.616	15	NC	1
560			min	-.013	2	-.629	2	0	1	0	1	129.662	2	4640.14	4
561		15	max	.018	3	.307	3	.234	4	5.93e-3	4	7341.332	15	NC	1
562			min	-.013	2	-.413	2	0	1	0	1	172.564	2	8127.37	4
563		16	max	.017	3	.154	3	.204	4	7.675e-3	4	NC	15	NC	1
564			min	-.012	2	-.201	2	0	1	0	1	255.102	2	NC	1
565		17	max	.017	3	.012	3	.176	4	9.42e-3	4	NC	5	NC	1
566			min	-.012	2	-.017	2	0	1	0	1	438.968	2	NC	1
567		18	max	.017	3	.125	1	.154	4	4.784e-3	4	NC	5	NC	1
568			min	-.012	2	-.109	3	0	1	0	1	966.025	1	NC	1
569		19	max	.017	3	.242	1	.137	4	0	1	NC	1	NC	1
570			min	-.012	2	-.219	3	0	1	-1.862e-6	4	NC	1	NC	1
571	M9	1	max	.007	3	.125	2	.511	4	2.528e-2	3	NC	1	NC	1
572			min	-.004	2	-.025	3	0	1	-1.383e-2	1	NC	1	NC	1
573		2	max	.007	3	.061	2	.499	4	1.251e-2	3	NC	4	NC	1
574			min	-.004	2	-.011	3	0	12	-6.726e-3	1	1783.195	2	NC	1
575		3	max	.007	3	.011	3	.485	4	1.273e-2	4	NC	5	NC	1
576			min	-.004	2	-.009	2	0	12	-3.351e-5	10	859.632	2	7044.967	4
577		4	max	.007	3	.049	3	.47	4	1.002e-2	5	NC	5	NC	1
578			min	-.004	2	-.087	2	0	12	-4.468e-3	2	542.84	2	5206.029	4
579		5	max	.007	3	.097	3	.454	4	9.535e-3	3	NC	5	NC	1
580			min	-.004	2	-.169	2	0	12	-8.915e-3	2	391.889	2	4276.894	4
581		6	max	.007	3	.149	3	.438	4	1.424e-2	3	NC	15	NC	1
582			min	-.004	2	-.248	2	0	12	-1.336e-2	2	308.719	2	3704.66	4
583		7	max	.007	3	.199	3	.421	4	1.894e-2	3	NC	15	NC	1
584			min	-.004	2	-.319	2	0	1	-1.781e-2	2	259.618	2	3296.67	4
585		8	max	.007	3	.241	3	.404	4	2.365e-2	3	9255.664	15	NC	1
586			min	-.003	2	-.375	2	0	1	-2.226e-2	2	230.572	2	2966.769	4
587		9	max	.007	3	.268	3	.386	4	2.385e-2	3	8654.113	15	NC	1
588			min	-.003	2	-.41	2	0	12	-2.524e-2	2	215.451	2	2719.224	4
589		10	max	.006	3	.278	3	.367	4	2.106e-2	3	8470.741	15	NC	1
590			min	-.003	2	-.422	2	0	1	-2.726e-2	2	211.013	2	2663.262	4
591		11	max	.006	3	.271	3	.345	4	1.827e-2	3	8653.843	15	NC	1
592			min	-.003	2	-.41	2	0	1	-2.928e-2	2	216.163	2	2730.169	4
593		12	max	.006	3	.248	3	.322	4	1.538e-2	3	9255.137	15	NC	1
594			min	-.003	2	-.373	2	0	12	-2.826e-2	2	232.733	2	2895.874	4
595		13	max	.006	3	.211	3	.295	4	1.231e-2	3	NC	15	NC	1
596			min	-.003	2	-.315	2	0	12	-2.266e-2	2	264.867	2	3428.872	4
597		14	max	.006	3	.164	3	.265	4	9.243e-3	3	NC	15	NC	1
598			min	-.003	2	-.242	2	-.002	1	-1.706e-2	2	319.912	2	4619.714	5
599		15	max	.006	3	.111	3	.234	4	6.177e-3	3	NC	5	NC	1
600			min	-.003	2	-.161	2	-.004	1	-1.146e-2	2	414.88	2	7442.831	5
601		16	max	.006	3	.057	3	.205	4	7.523e-3	5	NC	5	NC	1
602			min	-.003	2	-.08	2	-.006	1	-5.861e-3	2	591.211	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### ***Envelope Member Section Deflections (Continued)***

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.005	3	.004	3	.177	4	9.478e-3	4	NC	5	NC	1
604		min	-.003	2	-.006	2	-.006	1	-4.604e-4	1	969.077	2	NC	1
605	18	max	.005	3	.056	2	.155	4	4.539e-3	5	NC	4	NC	1
606		min	-.003	2	-.043	3	-.004	1	-9.982e-3	2	2061.473	2	NC	1
607	19	max	.005	3	.11	2	.137	4	8.447e-3	3	NC	1	NC	1
608		min	-.003	2	-.087	3	0	12	-2.006e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Software  
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Company:	Schletter, Inc.	Date:	11/17/2015
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Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 36 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

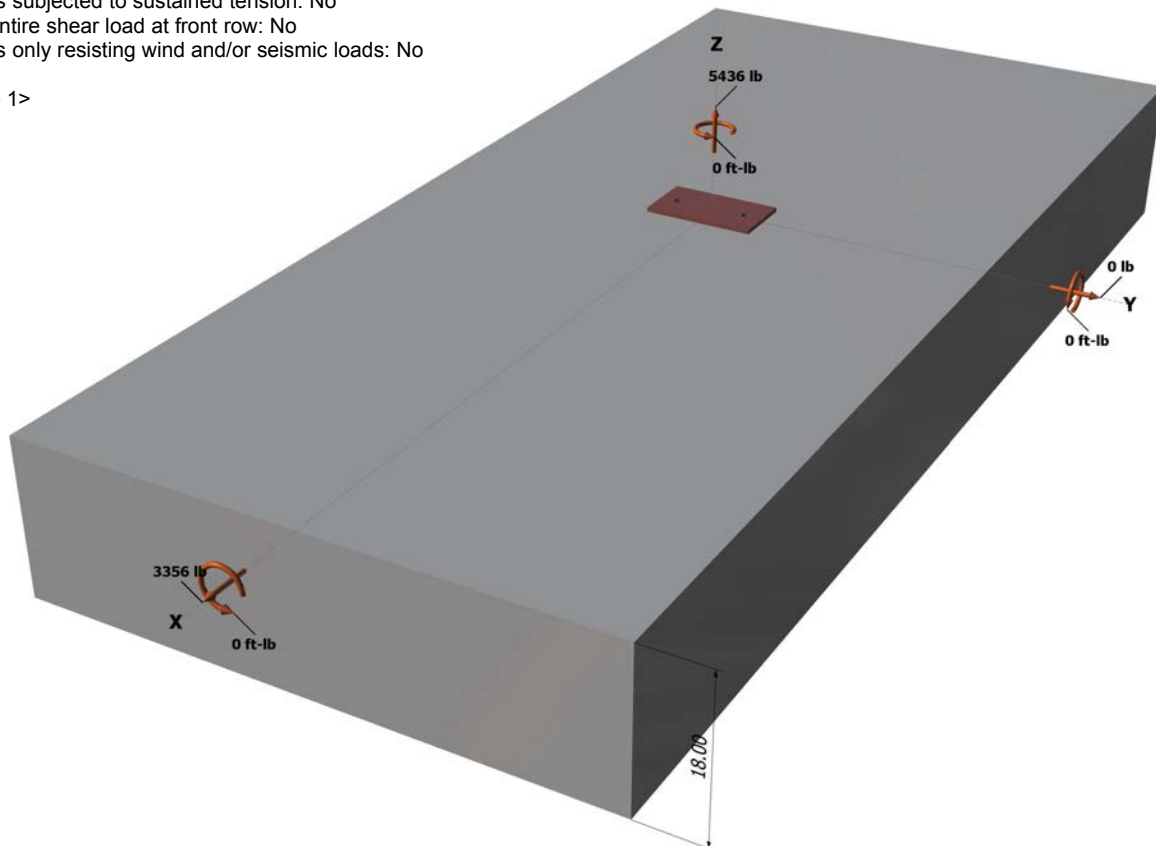
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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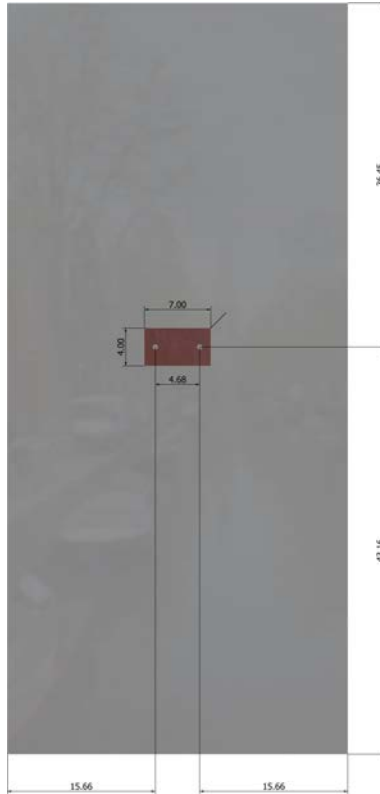




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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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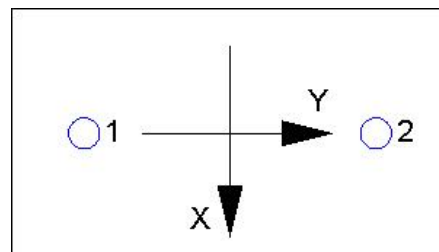
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2718.0	1678.0	0.0	1678.0
2	2718.0	1678.0	0.0	1678.0
Sum	5436.0	3356.0	0.0	3356.0

Maximum concrete compression strain (‰): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 5436  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

### Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
648.00	648.00	1.000	0.961	1.000	1.000	15593	0.70	10490

### Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	15.66	23247

$$\phi V_{cbx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
845.64	1103.56	1.000	1.000	1.000	23247	0.70	24939

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2718	6071	0.45	Pass	
Concrete breakout	5436	10231	0.53	Pass	
<b>Adhesive</b>	<b>5436</b>	<b>8093</b>	<b>0.67</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1678</b>	<b>3156</b>	<b>0.53</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3356	10490	0.32	Pass	
Concrete breakout y-	1678	24939	0.07	Pass	
Pryout	3356	20601	0.16	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.67	0.53	120.3 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.