

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-10	35° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	150 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 35.33 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.06	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	87 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.698 k-ft
$M_z$ =	0.002 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>61%</b>



DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

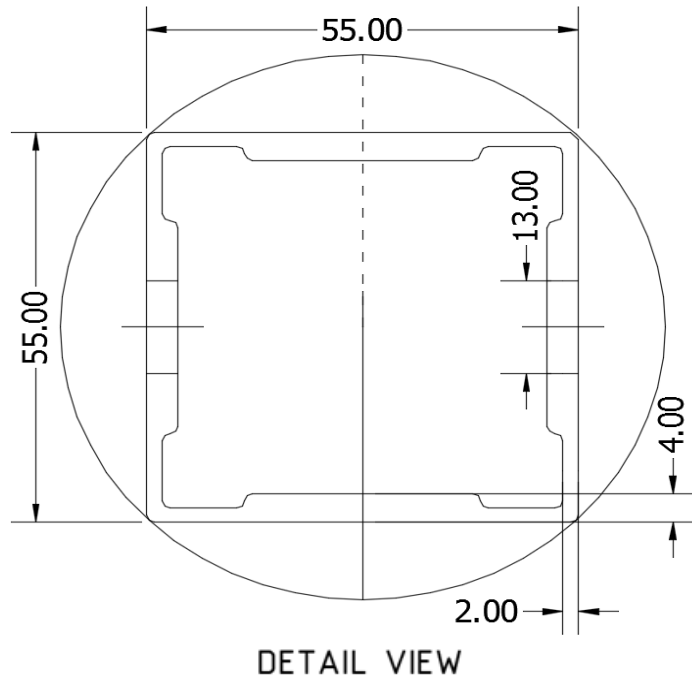
Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.692 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.981 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>79%</b>



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

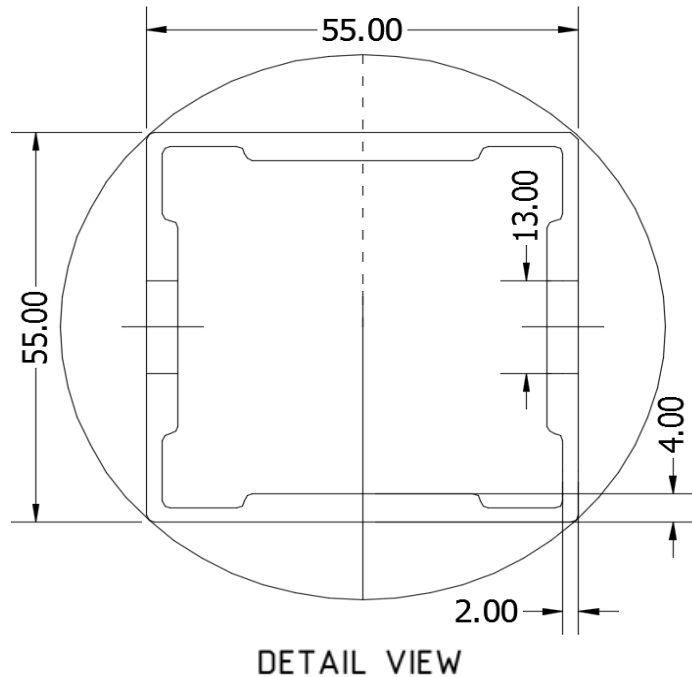
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	-0.415 k-ft
$P_n$ =	0.331 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>31%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.852 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>39%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.960 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>34%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

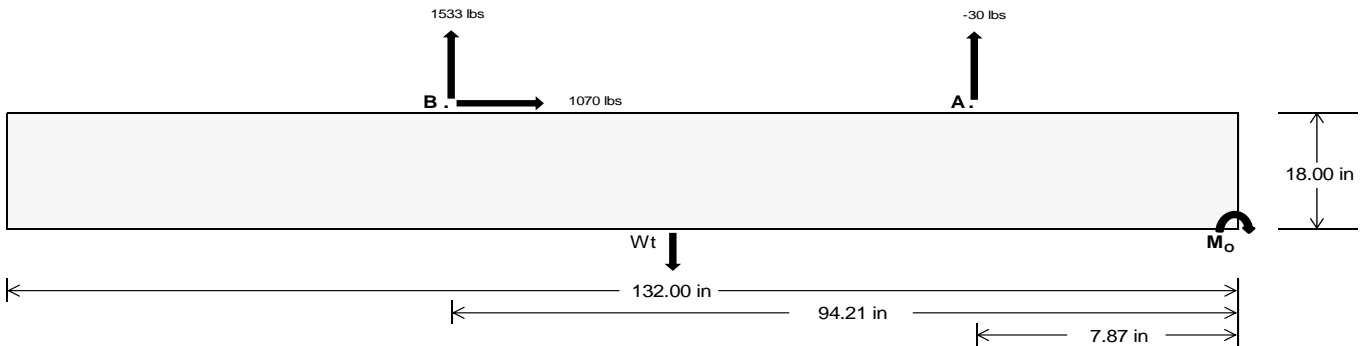
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>96.44</u>	<u>6653.07</u>	k
Compressive Load =	<u>2432.07</u>	<u>4820.18</u>	k
Lateral Load =	<u>298.11</u>	<u>4637.67</u>	k
Moment (Weak Axis) =	<u>0.55</u>	<u>0.15</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 163405.0$  in-lbs  
Resisting Force Required = 2475.83 lbs  
S.F. = 1.67  
Weight Required = 4126.39 lbs  
Minimum Width = 32 in  
Weight Provided = 6380.00 lbs

### Sliding

Force = 1070.08 lbs  
Friction = 0.4  
Weight Required = 2675.21 lbs  
Resisting Weight = 6380.00 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 1070.08 lbs  
Cohesion = 130 psf  
Area = 29.33 ft<sup>2</sup>  
Resisting = 3190.00 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 32in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 32in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 32in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

#### Ballast Width

$P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.67 \text{ ft}) =$  6380 lbs 6579 lbs 6779 lbs 6978 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in	32 in	33 in	34 in	35 in
$F_A$	786 lbs	786 lbs	786 lbs	786 lbs	1002 lbs	1002 lbs	1002 lbs	1002 lbs	1242 lbs	1242 lbs	1242 lbs	1242 lbs	61 lbs	61 lbs	61 lbs	61 lbs
$F_B$	692 lbs	692 lbs	692 lbs	692 lbs	2117 lbs	2117 lbs	2117 lbs	2117 lbs	2020 lbs	2020 lbs	2020 lbs	2020 lbs	-3065 lbs	-3065 lbs	-3065 lbs	-3065 lbs
$F_V$	112 lbs	112 lbs	112 lbs	112 lbs	1935 lbs	1935 lbs	1935 lbs	1935 lbs	1522 lbs	1522 lbs	1522 lbs	1522 lbs	-2140 lbs	-2140 lbs	-2140 lbs	-2140 lbs
$P_{total}$	7859 lbs	8058 lbs	8257 lbs	8457 lbs	9499 lbs	9698 lbs	9898 lbs	10097 lbs	9642 lbs	9841 lbs	10041 lbs	10240 lbs	824 lbs	943 lbs	1063 lbs	1182 lbs
$M$	2347 lbs-ft	2347 lbs-ft	2347 lbs-ft	2347 lbs-ft	2776 lbs-ft	2776 lbs-ft	2776 lbs-ft	2776 lbs-ft	3549 lbs-ft	3549 lbs-ft	3549 lbs-ft	3549 lbs-ft	4289 lbs-ft	4289 lbs-ft	4289 lbs-ft	4289 lbs-ft
$e$	0.30 ft	0.29 ft	0.28 ft	0.28 ft	0.29 ft	0.29 ft	0.28 ft	0.27 ft	0.37 ft	0.36 ft	0.35 ft	0.35 ft	5.21 ft	4.55 ft	4.04 ft	3.63 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	224.3 psf	224.1 psf	223.9 psf	223.7 psf	272.2 psf	270.6 psf	269.0 psf	267.5 psf	262.7 psf	261.3 psf	260.0 psf	258.8 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	311.5 psf	308.7 psf	306.0 psf	303.5 psf	375.4 psf	370.7 psf	366.2 psf	361.9 psf	394.7 psf	389.3 psf	384.3 psf	379.5 psf	705.5 psf	240.1 psf	170.8 psf	144.3 psf

Maximum Bearing Pressure = 706 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 32in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

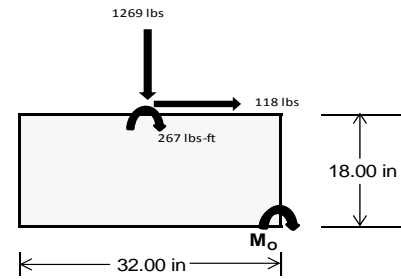
### Overturning Check

$M_o = 1246.8$  ft-lbs  
 Resisting Force Required = 935.11 lbs  
 S.F. = 1.67  
 Weight Required = 1558.52 lbs  
 Minimum Width = 32 in  
 Weight Provided = 6380.00 lbs

*A minimum 132in long x 32in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	32 in			32 in			32 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	262 lbs	461 lbs	139 lbs	540 lbs	1269 lbs	446 lbs	120 lbs	135 lbs	-3 lbs
$F_v$	163 lbs	159 lbs	165 lbs	120 lbs	118 lbs	127 lbs	163 lbs	160 lbs	164 lbs
$P_{total}$	8161 lbs	8359 lbs	8037 lbs	8059 lbs	8787 lbs	7965 lbs	2430 lbs	2444 lbs	2307 lbs
$M$	599 lbs-ft	591 lbs-ft	608 lbs-ft	445 lbs-ft	445 lbs-ft	467 lbs-ft	600 lbs-ft	589 lbs-ft	603 lbs-ft
$e$	0.07 ft	0.07 ft	0.08 ft	0.06 ft	0.05 ft	0.06 ft	0.25 ft	0.24 ft	0.26 ft
$L/6$	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft	0.44 ft
$f_{min}$	232.3 psf	239.6 psf	227.4 psf	240.6 psf	265.5 psf	235.7 psf	36.8 psf	38.2 psf	32.4 psf
$f_{max}$	324.1 psf	330.3 psf	320.6 psf	308.9 psf	333.7 psf	307.4 psf	128.8 psf	128.5 psf	124.9 psf



Maximum Bearing Pressure = 334 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 32in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 132in long x 32in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.901 k
Allowable Uplift =	1.214 k
Utilization =	<u>74%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.345 k
Allowable Uplift =	4.357 k
Utilization =	<u>54%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.871 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>25%</u>

#### Rear Strut

Maximum Axial Load =	4.394 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>59%</u>

#### Diagonal Strut

Maximum Axial Load =	2.910 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	$0.020h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u><math>0.448 \leq 1.076</math>, OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 87 \text{ in}$$

$$J = 0.432$$

$$240.683$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.3 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 87$$

$$J = 0.432$$

$$153.06$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL			.8			8		

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-118.221	-118.221	0	0
2	M14	y	-118.221	-118.221	0	0
3	M15	y	-197.035	-197.035	0	0
4	M16	y	-197.035	-197.035	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	265.997	265.997	0	0
2	M14	y	206.886	206.886	0	0
3	M15	y	118.221	118.221	0	0
4	M16	y	118.221	118.221	0	0

### Member Distributed Loads (BLC 6 : Seismic - Lateral)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Z	6.693	6.693	0	0
2	M14	Z	6.693	6.693	0	0
3	M15	Z	6.693	6.693	0	0
4	M16	Z	6.693	6.693	0	0
5	M13	Z	0	0	0	0
6	M14	Z	0	0	0	0
7	M15	Z	0	0	0	0
8	M16	Z	0	0	0	0



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 35° 150mph 30psf 7.25ft 7-10.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
19		10	max	54.858	4	1182.568	3	142.043	1	.002	14	.183	1	.993	2
20			min	5.946	12	-649.69	2	-92.203	14	-.013	2	.002	3	-1.698	3
21		11	max	53.285	1	535.763	2	-4.076	12	.013	2	.093	4	.516	2
22			min	5.946	12	-971.829	3	-112.197	1	0	15	-.004	3	-.831	3
23		12	max	53.285	1	421.836	2	-2.425	12	.013	2	.047	4	.13	2
24			min	5.946	12	-761.09	3	-82.35	1	0	15	-.008	3	-.133	3
25		13	max	53.285	1	307.909	2	-.775	12	.013	2	.022	5	.396	3
26			min	5.946	12	-550.35	3	-52.504	1	0	15	-.052	1	-.164	2
27		14	max	53.285	1	193.982	2	1.653	3	.013	2	0	15	.754	3
28			min	5.281	15	-339.611	3	-32.735	4	0	15	-.083	1	-.366	2
29		15	max	53.285	1	80.054	2	7.189	1	.013	2	-.005	12	.943	3
30			min	-.284	15	-128.872	3	-24.571	5	0	15	-.089	1	-.476	2
31		16	max	53.285	1	81.868	3	37.036	1	.013	2	-.002	12	.962	3
32			min	-8.536	5	-33.873	2	-22.017	5	0	15	-.071	1	-.495	2
33		17	max	53.285	1	292.607	3	66.882	1	.013	2	.004	3	.811	3
34			min	-16.805	5	-147.8	2	-19.463	5	0	15	-.065	4	-.422	2
35		18	max	53.285	1	503.346	3	96.729	1	.013	2	.037	1	.49	3
36			min	-25.074	5	-261.727	2	-16.909	5	0	15	-.072	5	-.257	2
37		19	max	53.285	1	714.085	3	126.575	1	.013	2	.127	1	0	2
38			min	-33.343	5	-375.655	2	-14.355	5	0	15	-.085	5	0	3
39	M14	1	max	34.78	4	431.637	2	-9.438	12	.011	3	.204	4	0	4
40			min	2.79	12	-589.765	3	-131.638	1	-.012	2	.016	12	0	3
41		2	max	30.35	1	317.71	2	-7.787	12	.011	3	.139	4	.409	3
42			min	2.79	12	-426.643	3	-101.791	1	-.012	2	.005	10	-.302	2
43		3	max	30.35	1	203.783	2	-6.136	12	.011	3	.083	5	.687	3
44			min	2.79	12	-263.521	3	-71.945	1	-.012	2	-.013	1	-.512	2
45		4	max	30.35	1	89.856	2	-4.486	12	.011	3	.046	5	.834	3
46			min	1.462	15	-100.399	3	-56.06	4	-.012	2	-.059	1	-.63	2
47		5	max	30.35	1	62.723	3	-.431	10	.011	3	.011	5	.849	3
48			min	-6.057	5	-24.072	2	-45.99	4	-.012	2	-.081	1	-.657	2
49		6	max	30.35	1	225.845	3	17.595	1	.011	3	-.005	12	.733	3
50			min	-14.326	5	-137.999	2	-39.604	5	-.012	2	-.079	1	-.591	2
51		7	max	30.35	1	388.967	3	47.441	1	.011	3	-.006	12	.485	3
52			min	-22.596	5	-251.926	2	-37.05	5	-.012	2	-.067	4	-.434	2
53		8	max	30.35	1	552.09	3	77.288	1	.011	3	.003	2	.106	3
54			min	-30.865	5	-365.853	2	-34.496	5	-.012	2	-.083	4	-.185	2
55		9	max	30.35	1	715.212	3	107.134	1	.011	3	.072	1	.155	2
56			min	-39.134	5	-479.781	2	-31.942	5	-.012	2	-.108	5	-.404	3
57		10	max	60.359	4	878.334	3	136.981	1	.011	3	.204	4	.588	2
58			min	2.79	12	-593.708	2	-97.388	14	-.012	2	.001	3	-1.046	3
59		11	max	52.09	4	479.781	2	-3.768	12	.012	2	.138	4	.155	2
60			min	2.79	12	-715.212	3	-107.134	1	-.011	3	-.004	3	-.404	3
61		12	max	43.821	4	365.853	2	-2.117	12	.012	2	.08	5	.106	3
62			min	2.79	12	-552.09	3	-77.288	1	-.011	3	-.008	3	-.185	2
63		13	max	35.552	4	251.926	2	-.357	3	.012	2	.043	5	.485	3
64			min	2.79	12	-388.967	3	-56.934	4	-.011	3	-.053	1	-.434	2
65		14	max	30.35	1	137.999	2	2.119	3	.012	2	.008	5	.733	3
66			min	2.79	12	-225.845	3	-46.864	4	-.011	3	-.079	1	-.591	2
67		15	max	30.35	1	24.072	2	12.252	1	.012	2	-.004	12	.849	3
68			min	2.79	12	-62.723	3	-39.826	5	-.011	3	-.081	1	-.657	2
69		16	max	30.35	1	100.399	3	42.098	1	.012	2	0	3	.834	3
70			min	1.843	15	-89.856	2	-37.272	5	-.011	3	-.071	4	-.63	2
71		17	max	30.35	1	263.521	3	71.945	1	.012	2	.006	3	.687	3
72			min	-5.467	5	-203.783	2	-34.718	5	-.011	3	-.088	4	-.512	2
73		18	max	30.35	1	426.643	3	101.791	1	.012	2	.057	1	.409	3
74			min	-13.736	5	-317.71	2	-32.164	5	-.011	3	-.112	5	-.302	2
75		19	max	30.35	1	589.765	3	131.638	1	.012	2	.151	1	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
76			min	-22.005	5	-431.637	2	-29.61	5	-.011	3	-.137	5	0	3
77	M15	1	max	65.906	5	645.972	2	-9.265	12	.013	2	.252	4	0	2
78			min	-31.255	1	-349.024	3	-131.671	1	-.01	3	.015	12	0	3
79		2	max	57.637	5	468.556	2	-7.614	12	.013	2	.176	4	.244	3
80			min	-31.255	1	-257.326	3	-101.825	1	-.01	3	.005	10	-.449	2
81		3	max	49.368	5	291.14	2	-5.963	12	.013	2	.11	5	.415	3
82			min	-31.255	1	-165.629	3	-79.012	4	-.01	3	-.013	1	-.755	2
83		4	max	41.099	5	113.724	2	-4.313	12	.013	2	.063	5	.511	3
84			min	-31.255	1	-73.931	3	-68.942	4	-.01	3	-.059	1	-.918	2
85		5	max	32.83	5	17.766	3	-.518	10	.013	2	.017	5	.534	3
86			min	-31.255	1	-63.693	2	-58.872	4	-.01	3	-.081	1	-.938	2
87		6	max	24.56	5	109.464	3	17.561	1	.013	2	-.005	12	.482	3
88			min	-31.255	1	-241.109	2	-52.444	5	-.01	3	-.079	1	-.815	2
89		7	max	16.291	5	201.161	3	47.408	1	.013	2	-.005	12	.357	3
90			min	-31.255	1	-418.525	2	-49.891	5	-.01	3	-.081	4	-.55	2
91		8	max	8.022	5	292.859	3	77.254	1	.013	2	.003	2	.158	3
92			min	-31.255	1	-595.941	2	-47.337	5	-.01	3	-.109	4	-.141	2
93		9	max	-.061	15	384.557	3	107.101	1	.013	2	.072	1	.41	2
94			min	-31.255	1	-773.358	2	-44.783	5	-.01	3	-.143	5	-.114	3
95		10	max	-3.574	10	476.254	3	136.947	1	.013	2	.25	4	1.105	2
96			min	-31.255	1	-950.774	2	-103.941	14	-.01	3	.002	3	-.461	3
97		11	max	-3.574	10	773.358	2	-3.94	12	.01	3	.174	4	.41	2
98			min	-31.255	1	-384.557	3	-107.101	1	-.013	2	-.003	3	-.114	3
99		12	max	-3.574	10	595.941	2	-2.29	12	.01	3	.105	5	.158	3
100			min	-31.255	1	-292.859	3	-79.915	4	-.013	2	-.007	3	-.141	2
101		13	max	-3.574	10	418.525	2	-.639	12	.01	3	.058	5	.357	3
102			min	-31.255	1	-201.161	3	-69.845	4	-.013	2	-.053	1	-.55	2
103		14	max	-3.574	10	241.109	2	1.832	3	.01	3	.012	5	.482	3
104			min	-39.093	4	-109.464	3	-59.775	4	-.013	2	-.079	1	-.815	2
105		15	max	-3.574	10	63.693	2	12.285	1	.01	3	-.004	12	.534	3
106			min	-47.362	4	-17.766	3	-52.673	5	-.013	2	-.081	1	-.938	2
107		16	max	-3.574	10	73.931	3	42.132	1	.01	3	0	12	.511	3
108			min	-55.631	4	-113.724	2	-50.119	5	-.013	2	-.088	4	-.918	2
109		17	max	-3.574	10	165.629	3	71.978	1	.01	3	.005	3	.415	3
110			min	-63.901	4	-291.14	2	-47.565	5	-.013	2	-.115	4	-.755	2
111		18	max	-3.574	10	257.326	3	101.825	1	.01	3	.057	1	.244	3
112			min	-72.17	4	-468.556	2	-45.011	5	-.013	2	-.149	5	-.449	2
113		19	max	-3.574	10	349.024	3	131.671	1	.01	3	.151	1	0	2
114			min	-80.439	4	-645.972	2	-42.457	5	-.013	2	-.184	5	0	5
115	M16	1	max	63.809	5	592.722	2	-8.558	12	.007	2	.194	4	0	2
116			min	-58.095	1	-300.357	3	-126.984	1	-.012	3	.012	12	0	3
117		2	max	55.54	5	415.306	2	-6.907	12	.007	2	.131	4	.205	3
118			min	-58.095	1	-208.66	3	-97.137	1	-.012	3	.003	10	-.406	2
119		3	max	47.27	5	237.89	2	-5.257	12	.007	2	.083	5	.336	3
120			min	-58.095	1	-116.962	3	-67.291	1	-.012	3	-.028	1	-.669	2
121		4	max	39.001	5	60.474	2	-3.606	12	.007	2	.048	5	.393	3
122			min	-58.095	1	-25.265	3	-52.358	4	-.012	3	-.07	1	-.789	2
123		5	max	30.732	5	66.433	3	-.059	10	.007	2	.015	5	.377	3
124			min	-58.095	1	-116.943	2	-42.289	4	-.012	3	-.089	1	-.767	2
125		6	max	22.463	5	158.13	3	22.249	1	.007	2	-.005	12	.286	3
126			min	-58.095	1	-294.359	2	-37.218	5	-.012	3	-.083	1	-.601	2
127		7	max	14.194	5	249.828	3	52.095	1	.007	2	-.005	12	.122	3
128			min	-58.095	1	-471.775	2	-34.664	5	-.012	3	-.059	4	-.292	2
129		8	max	5.924	5	341.525	3	81.942	1	.007	2	.004	2	.159	2
130			min	-58.095	1	-649.192	2	-32.11	5	-.012	3	-.073	4	-.116	3
131		9	max	-1.456	15	433.223	3	111.788	1	.007	2	.079	1	.754	2
132			min	-58.095	1	-826.608	2	-29.556	5	-.012	3	-.097	5	-.428	3



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
133	10	max	-5.662	12	524.92	3	141.635	1	.007	2	.196	4	1.491	2
134		min	-58.095	1	-1004.024	2	-99.144	14	-.012	3	.004	12	-.814	3
135	11	max	-5.232	15	826.608	2	-4.647	12	.012	3	.13	4	.754	2
136		min	-58.095	1	-433.223	3	-111.788	1	-.007	2	-.001	3	-.428	3
137	12	max	-5.662	12	649.192	2	-2.997	12	.012	3	.072	4	.159	2
138		min	-58.095	1	-341.525	3	-81.942	1	-.007	2	-.006	3	-.116	3
139	13	max	-5.662	12	471.775	2	-1.346	12	.012	3	.036	5	.122	3
140		min	-58.095	1	-249.828	3	-56.593	4	-.007	2	-.053	1	-.292	2
141	14	max	-5.662	12	294.359	2	.7	3	.012	3	.002	5	.286	3
142		min	-58.095	1	-158.13	3	-46.523	4	-.007	2	-.083	1	-.601	2
143	15	max	-5.662	12	116.943	2	7.598	1	.012	3	-.005	12	.377	3
144		min	-58.095	1	-66.433	3	-38.294	5	-.007	2	-.089	1	-.767	2
145	16	max	-5.662	12	25.265	3	37.444	1	.012	3	-.002	12	.393	3
146		min	-64.822	4	-60.474	2	-35.74	5	-.007	2	-.078	4	-.789	2
147	17	max	-5.662	12	116.962	3	67.291	1	.012	3	.002	3	.336	3
148		min	-73.092	4	-237.89	2	-33.187	5	-.007	2	-.095	4	-.669	2
149	18	max	-5.662	12	208.66	3	97.137	1	.012	3	.038	1	.205	3
150		min	-81.361	4	-415.306	2	-30.633	5	-.007	2	-.113	5	-.406	2
151	19	max	-5.662	12	300.357	3	126.984	1	.012	3	.128	1	0	2
152		min	-89.63	4	-592.722	2	-28.079	5	-.007	2	-.137	5	0	5
153	M2	1	max	958.183	2	2.044	4	.179	1	0	3	0	3	1
154		min	-1392.854	3	.491	15	-15.032	4	0	4	0	2	0	1
155	2	max	958.704	2	1.925	4	.179	1	0	3	0	1	0	15
156		min	-1392.464	3	.463	15	-15.49	4	0	4	-.005	4	0	4
157	3	max	959.224	2	1.806	4	.179	1	0	3	0	1	0	15
158		min	-1392.073	3	.435	15	-15.948	4	0	4	-.011	4	-.001	4
159	4	max	959.745	2	1.687	4	.179	1	0	3	0	1	0	15
160		min	-1391.683	3	.407	15	-16.407	4	0	4	-.017	4	-.002	4
161	5	max	960.266	2	1.568	4	.179	1	0	3	0	1	0	15
162		min	-1391.292	3	.379	15	-16.865	4	0	4	-.023	4	-.003	4
163	6	max	960.787	2	1.449	4	.179	1	0	3	0	1	0	15
164		min	-1390.902	3	.351	15	-17.324	4	0	4	-.029	4	-.003	4
165	7	max	961.307	2	1.331	4	.179	1	0	3	0	1	0	15
166		min	-1390.511	3	.323	15	-17.782	4	0	4	-.035	4	-.004	4
167	8	max	961.828	2	1.212	4	.179	1	0	3	0	1	0	15
168		min	-1390.121	3	.294	12	-18.24	4	0	4	-.042	4	-.004	4
169	9	max	962.349	2	1.093	4	.179	1	0	3	0	1	-.001	15
170		min	-1389.73	3	.248	12	-18.699	4	0	4	-.048	4	-.004	4
171	10	max	962.869	2	.974	4	.179	1	0	3	0	1	-.001	15
172		min	-1389.34	3	.202	12	-19.157	4	0	4	-.055	4	-.005	4
173	11	max	963.39	2	.855	4	.179	1	0	3	0	1	-.001	15
174		min	-1388.949	3	.156	12	-19.615	4	0	4	-.062	4	-.005	4
175	12	max	963.911	2	.76	2	.179	1	0	3	0	1	-.001	15
176		min	-1388.558	3	.109	12	-20.074	4	0	4	-.069	4	-.005	4
177	13	max	964.431	2	.668	2	.179	1	0	3	0	1	-.001	15
178		min	-1388.168	3	.063	12	-20.532	4	0	4	-.076	4	-.006	4
179	14	max	964.952	2	.575	2	.179	1	0	3	0	1	-.001	15
180		min	-1387.777	3	-.003	3	-20.99	4	0	4	-.083	4	-.006	4
181	15	max	965.473	2	.482	2	.179	1	0	3	0	1	-.001	12
182		min	-1387.387	3	-.073	3	-21.449	4	0	4	-.091	4	-.006	4
183	16	max	965.994	2	.39	2	.179	1	0	3	0	1	-.001	12
184		min	-1386.996	3	-.142	3	-21.907	4	0	4	-.099	4	-.006	4
185	17	max	966.514	2	.297	2	.179	1	0	3	.001	1	-.001	12
186		min	-1386.606	3	-.212	3	-22.365	4	0	4	-.107	4	-.006	4
187	18	max	967.035	2	.204	2	.179	1	0	3	.001	1	-.001	12
188		min	-1386.215	3	-.281	3	-22.824	4	0	4	-.115	4	-.006	4
189	19	max	967.556	2	.112	2	.179	1	0	3	.001	1	-.001	12





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
190		min	-1385.825	3	-.351	3	-23.282	4	0	4	-.123	4	-.006	4
191	M3	1	max	860.209	2	7.684	4	5.135	4	0	3	0	.006	4
192		min	-958.858	3	1.815	15	.018	12	0	4	-.022	4	.001	12
193		2	max	860.039	2	6.923	4	5.67	4	0	3	0	.004	2
194		min	-958.985	3	1.636	15	.018	12	0	4	-.02	4	0	3
195		3	max	859.868	2	6.162	4	6.204	4	0	3	0	.001	2
196		min	-959.113	3	1.458	15	.018	12	0	4	-.018	4	-.001	3
197		4	max	859.698	2	5.401	4	6.739	4	0	3	0	0	15
198		min	-959.241	3	1.279	15	.018	12	0	4	-.015	5	-.003	3
199		5	max	859.528	2	4.64	4	7.274	4	0	3	0	0	15
200		min	-959.369	3	1.1	15	.018	12	0	4	-.012	5	-.004	6
201		6	max	859.357	2	3.879	4	7.808	4	0	3	0	1	15
202		min	-959.496	3	.921	15	.018	12	0	4	-.009	5	-.006	6
203		7	max	859.187	2	3.118	4	8.343	4	0	3	0	1	15
204		min	-959.624	3	.742	15	.018	12	0	4	-.006	5	-.007	6
205		8	max	859.017	2	2.357	4	8.878	4	0	3	0	1	15
206		min	-959.752	3	.563	15	.018	12	0	4	-.002	5	-.008	6
207		9	max	858.846	2	1.596	4	9.412	4	0	3	.002	4	15
208		min	-959.88	3	.384	15	.018	12	0	4	0	12	-.009	6
209		10	max	858.676	2	.835	4	9.947	4	0	3	.006	4	15
210		min	-960.007	3	.173	12	.018	12	0	4	0	12	-.01	6
211		11	max	858.506	2	.217	2	10.482	4	0	3	.01	4	15
212		min	-960.135	3	-.209	3	.018	12	0	4	0	12	-.01	6
213		12	max	858.335	2	-.152	15	11.017	4	0	3	.015	4	15
214		min	-960.263	3	-.688	6	.018	12	0	4	0	12	-.01	6
215		13	max	858.165	2	-.331	15	11.551	4	0	3	.019	4	15
216		min	-960.391	3	-1.449	6	.018	12	0	4	0	12	-.009	6
217		14	max	857.995	2	-.51	15	12.086	4	0	3	.024	4	15
218		min	-960.518	3	-2.21	6	.018	12	0	4	0	12	-.009	6
219		15	max	857.824	2	-.689	15	12.621	4	0	3	.029	4	15
220		min	-960.646	3	-2.971	6	.018	12	0	4	0	12	-.007	6
221		16	max	857.654	2	-.868	15	13.155	4	0	3	.035	4	15
222		min	-960.774	3	-3.732	6	.018	12	0	4	0	12	-.006	6
223		17	max	857.484	2	-1.047	15	13.69	4	0	3	.04	4	15
224		min	-960.902	3	-4.493	6	.018	12	0	4	0	12	-.004	6
225		18	max	857.313	2	-1.226	15	14.225	4	0	3	.046	4	15
226		min	-961.03	3	-5.254	6	.018	12	0	4	0	12	-.002	6
227		19	max	857.143	2	-1.404	15	14.759	4	0	3	.052	4	1
228		min	-961.157	3	-6.015	6	.018	12	0	4	0	12	0	1
229	M4	1	max	730.655	1	0	1	-.585	12	0	1	.049	4	1
230		min	-75.612	5	0	1	-227.082	4	0	1	0	12	0	1
231		2	max	730.825	1	0	1	-.585	12	0	1	.023	4	1
232		min	-75.532	5	0	1	-227.23	4	0	1	0	12	0	1
233		3	max	730.995	1	0	1	-.585	12	0	1	0	1	1
234		min	-75.453	5	0	1	-227.378	4	0	1	-.003	4	0	1
235		4	max	731.166	1	0	1	-.585	12	0	1	0	12	1
236		min	-75.373	5	0	1	-227.525	4	0	1	-.029	4	0	1
237		5	max	731.336	1	0	1	-.585	12	0	1	0	12	1
238		min	-75.294	5	0	1	-227.673	4	0	1	-.055	4	0	1
239		6	max	731.506	1	0	1	-.585	12	0	1	0	12	1
240		min	-75.214	5	0	1	-227.82	4	0	1	-.081	4	0	1
241		7	max	731.677	1	0	1	-.585	12	0	1	0	12	1
242		min	-75.135	5	0	1	-227.968	4	0	1	-.107	4	0	1
243		8	max	731.847	1	0	1	-.585	12	0	1	0	12	1
244		min	-75.055	5	0	1	-228.116	4	0	1	-.134	4	0	1
245		9	max	732.017	1	0	1	-.585	12	0	1	0	12	1
246		min	-74.976	5	0	1	-228.263	4	0	1	-.16	4	0	1





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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
247		10	max	732.188	1	0	1	-585	12	0	1	0	12	0	1
248			min	-74.896	5	0	1	-228.411	4	0	1	-.186	4	0	1
249		11	max	732.358	1	0	1	-585	12	0	1	0	12	0	1
250			min	-74.817	5	0	1	-228.559	4	0	1	-.212	4	0	1
251		12	max	732.528	1	0	1	-585	12	0	1	0	12	0	1
252			min	-74.737	5	0	1	-228.706	4	0	1	-.238	4	0	1
253		13	max	732.699	1	0	1	-585	12	0	1	0	12	0	1
254			min	-74.658	5	0	1	-228.854	4	0	1	-.265	4	0	1
255		14	max	732.869	1	0	1	-585	12	0	1	0	12	0	1
256			min	-74.578	5	0	1	-229.001	4	0	1	-.291	4	0	1
257		15	max	733.039	1	0	1	-585	12	0	1	0	12	0	1
258			min	-74.499	5	0	1	-229.149	4	0	1	-.317	4	0	1
259		16	max	733.21	1	0	1	-585	12	0	1	0	12	0	1
260			min	-74.419	5	0	1	-229.297	4	0	1	-.344	4	0	1
261		17	max	733.38	1	0	1	-585	12	0	1	0	12	0	1
262			min	-74.34	5	0	1	-229.444	4	0	1	-.37	4	0	1
263		18	max	733.55	1	0	1	-585	12	0	1	0	12	0	1
264			min	-74.26	5	0	1	-229.592	4	0	1	-.396	4	0	1
265		19	max	733.721	1	0	1	-585	12	0	1	-.001	12	0	1
266			min	-74.181	5	0	1	-229.74	4	0	1	-.423	4	0	1
267	M6	1	max	2950.168	2	2.238	2	0	1	0	1	0	4	0	1
268			min	-4394.305	3	.268	12	-15.19	4	0	4	0	1	0	1
269		2	max	2950.689	2	2.145	2	0	1	0	1	0	1	0	12
270			min	-4393.915	3	.221	12	-15.649	4	0	4	-.005	4	0	2
271		3	max	2951.21	2	2.053	2	0	1	0	1	0	1	0	12
272			min	-4393.524	3	.175	12	-16.107	4	0	4	-.011	4	-.002	2
273		4	max	2951.73	2	1.96	2	0	1	0	1	0	1	0	12
274			min	-4393.133	3	.106	3	-16.565	4	0	4	-.017	4	-.002	2
275		5	max	2952.251	2	1.867	2	0	1	0	1	0	1	0	12
276			min	-4392.743	3	.037	3	-17.024	4	0	4	-.023	4	-.003	2
277		6	max	2952.772	2	1.775	2	0	1	0	1	0	1	0	3
278			min	-4392.352	3	-.033	3	-17.482	4	0	4	-.029	4	-.004	2
279		7	max	2953.292	2	1.682	2	0	1	0	1	0	1	0	3
280			min	-4391.962	3	-.102	3	-17.94	4	0	4	-.035	4	-.004	2
281		8	max	2953.813	2	1.59	2	0	1	0	1	0	1	0	3
282			min	-4391.571	3	-.172	3	-18.399	4	0	4	-.042	4	-.005	2
283		9	max	2954.334	2	1.497	2	0	1	0	1	0	1	0	3
284			min	-4391.181	3	-.241	3	-18.857	4	0	4	-.049	4	-.005	2
285		10	max	2954.854	2	1.404	2	0	1	0	1	0	1	0	3
286			min	-4390.79	3	-.311	3	-19.315	4	0	4	-.055	4	-.006	2
287		11	max	2955.375	2	1.312	2	0	1	0	1	0	1	0	3
288			min	-4390.4	3	-.38	3	-19.774	4	0	4	-.062	4	-.006	2
289		12	max	2955.896	2	1.219	2	0	1	0	1	0	1	0	3
290			min	-4390.009	3	-.449	3	-20.232	4	0	4	-.069	4	-.007	2
291		13	max	2956.417	2	1.127	2	0	1	0	1	0	1	0	3
292			min	-4389.619	3	-.519	3	-20.69	4	0	4	-.077	4	-.007	2
293		14	max	2956.937	2	1.034	2	0	1	0	1	0	1	0	3
294			min	-4389.228	3	-.588	3	-21.149	4	0	4	-.084	4	-.008	2
295		15	max	2957.458	2	.941	2	0	1	0	1	0	1	0	3
296			min	-4388.838	3	-.658	3	-21.607	4	0	4	-.092	4	-.008	2
297		16	max	2957.979	2	.849	2	0	1	0	1	0	1	.001	3
298			min	-4388.447	3	-.727	3	-22.066	4	0	4	-.1	4	-.008	2
299		17	max	2958.499	2	.756	2	0	1	0	1	0	1	.001	3
300			min	-4388.057	3	-.797	3	-22.524	4	0	4	-.108	4	-.009	2
301		18	max	2959.02	2	.663	2	0	1	0	1	0	1	.002	3
302			min	-4387.666	3	-.866	3	-22.982	4	0	4	-.116	4	-.009	2
303		19	max	2959.541	2	.571	2	0	1	0	1	0	1	.002	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
304			min	-4387.276	3	-.936	3	-23.441	4	0	4	-.124	4	-.009	2
305	M7	1	max	2852.254	2	7.682	6	4.827	4	0	1	0	1	.009	2
306			min	-2907.37	3	1.805	15	0	1	0	4	-.023	4	-.002	3
307		2	max	2852.083	2	6.921	6	5.362	4	0	1	0	1	.006	2
308			min	-2907.498	3	1.626	15	0	1	0	4	-.02	4	-.003	3
309		3	max	2851.913	2	6.16	6	5.896	4	0	1	0	1	.004	2
310			min	-2907.625	3	1.447	15	0	1	0	4	-.018	4	-.005	3
311		4	max	2851.743	2	5.399	6	6.431	4	0	1	0	1	.002	2
312			min	-2907.753	3	1.268	15	0	1	0	4	-.016	4	-.006	3
313		5	max	2851.572	2	4.638	6	6.966	4	0	1	0	1	0	2
314			min	-2907.881	3	1.089	15	0	1	0	4	-.013	4	-.007	3
315		6	max	2851.402	2	3.877	6	7.501	4	0	1	0	1	-.001	15
316			min	-2908.009	3	.91	15	0	1	0	4	-.01	4	-.007	3
317		7	max	2851.231	2	3.116	6	8.035	4	0	1	0	1	-.002	15
318			min	-2908.136	3	.731	15	0	1	0	4	-.007	5	-.008	3
319		8	max	2851.061	2	2.39	2	8.57	4	0	1	0	1	-.002	15
320			min	-2908.264	3	.465	12	0	1	0	4	-.003	5	-.008	4
321		9	max	2850.891	2	1.797	2	9.105	4	0	1	0	4	-.002	15
322			min	-2908.392	3	.169	12	0	1	0	4	0	1	-.009	4
323		10	max	2850.72	2	1.204	2	9.639	4	0	1	.004	4	-.002	15
324			min	-2908.52	3	-.266	3	0	1	0	4	0	1	-.01	4
325		11	max	2850.55	2	.611	2	10.174	4	0	1	.009	4	-.002	15
326			min	-2908.647	3	-.711	3	0	1	0	4	0	1	-.01	4
327		12	max	2850.38	2	.018	2	10.709	4	0	1	.013	4	-.002	15
328			min	-2908.775	3	-1.156	3	0	1	0	4	0	1	-.01	4
329		13	max	2850.209	2	-.342	15	11.243	4	0	1	.018	4	-.002	15
330			min	-2908.903	3	-1.6	3	0	1	0	4	0	1	-.009	4
331		14	max	2850.039	2	-.521	15	11.778	4	0	1	.022	4	-.002	15
332			min	-2909.031	3	-2.211	4	0	1	0	4	0	1	-.009	4
333		15	max	2849.869	2	-.7	15	12.313	4	0	1	.027	4	-.002	15
334			min	-2909.159	3	-2.972	4	0	1	0	4	0	1	-.007	4
335		16	max	2849.698	2	-.879	15	12.847	4	0	1	.033	4	-.001	15
336			min	-2909.286	3	-3.733	4	0	1	0	4	0	1	-.006	4
337		17	max	2849.528	2	-1.057	15	13.382	4	0	1	.038	4	-.001	15
338			min	-2909.414	3	-4.494	4	0	1	0	4	0	1	-.004	4
339		18	max	2849.358	2	-1.236	15	13.917	4	0	1	.044	4	0	15
340			min	-2909.542	3	-5.255	4	0	1	0	4	0	1	-.002	4
341		19	max	2849.187	2	-1.415	15	14.452	4	0	1	.05	4	0	1
342			min	-2909.67	3	-6.016	4	0	1	0	4	0	1	0	1
343	M8	1	max	1867.759	2	0	1	0	1	0	1	.047	4	0	1
344			min	70.304	15	0	1	-219.283	4	0	1	0	1	0	1
345		2	max	1867.93	2	0	1	0	1	0	1	.022	4	0	1
346			min	70.356	15	0	1	-219.431	4	0	1	0	1	0	1
347		3	max	1868.1	2	0	1	0	1	0	1	0	1	0	1
348			min	70.407	15	0	1	-219.579	4	0	1	-.003	4	0	1
349		4	max	1868.27	2	0	1	0	1	0	1	0	1	0	1
350			min	70.458	15	0	1	-219.726	4	0	1	-.029	4	0	1
351		5	max	1868.441	2	0	1	0	1	0	1	0	1	0	1
352			min	70.51	15	0	1	-219.874	4	0	1	-.054	4	0	1
353		6	max	1868.611	2	0	1	0	1	0	1	0	1	0	1
354			min	70.561	15	0	1	-220.021	4	0	1	-.079	4	0	1
355		7	max	1868.781	2	0	1	0	1	0	1	0	1	0	1
356			min	70.612	15	0	1	-220.169	4	0	1	-.104	4	0	1
357		8	max	1868.952	2	0	1	0	1	0	1	0	1	0	1
358			min	70.664	15	0	1	-220.317	4	0	1	-.13	4	0	1
359		9	max	1869.122	2	0	1	0	1	0	1	0	1	0	1
360			min	70.715	15	0	1	-220.464	4	0	1	-.155	4	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
361		10	max	1869.292	2	0	1	0	1	0	1	0	1	0	1
362			min	70.767	15	0	1	-220.612	4	0	1	-.18	4	0	1
363		11	max	1869.463	2	0	1	0	1	0	1	0	1	0	1
364			min	70.818	15	0	1	-220.76	4	0	1	-.206	4	0	1
365		12	max	1869.633	2	0	1	0	1	0	1	0	1	0	1
366			min	70.869	15	0	1	-220.907	4	0	1	-.231	4	0	1
367		13	max	1869.803	2	0	1	0	1	0	1	0	1	0	1
368			min	70.921	15	0	1	-221.055	4	0	1	-.256	4	0	1
369		14	max	1869.974	2	0	1	0	1	0	1	0	1	0	1
370			min	70.972	15	0	1	-221.203	4	0	1	-.282	4	0	1
371		15	max	1870.144	2	0	1	0	1	0	1	0	1	0	1
372			min	71.024	15	0	1	-221.35	4	0	1	-.307	4	0	1
373		16	max	1870.315	2	0	1	0	1	0	1	0	1	0	1
374			min	71.075	15	0	1	-221.498	4	0	1	-.333	4	0	1
375		17	max	1870.485	2	0	1	0	1	0	1	0	1	0	1
376			min	71.126	15	0	1	-221.645	4	0	1	-.358	4	0	1
377		18	max	1870.655	2	0	1	0	1	0	1	0	1	0	1
378			min	71.178	15	0	1	-221.793	4	0	1	-.383	4	0	1
379		19	max	1870.826	2	0	1	0	1	0	1	0	1	0	1
380			min	71.229	15	0	1	-221.941	4	0	1	-.409	4	0	1
381	M10	1	max	958.183	2	1.996	6	-.019	12	0	1	0	4	0	1
382			min	-1392.854	3	.459	15	-15.157	4	0	5	0	3	0	1
383		2	max	958.704	2	1.877	6	-.019	12	0	1	0	10	0	15
384			min	-1392.464	3	.431	15	-15.615	4	0	5	-.005	4	0	6
385		3	max	959.224	2	1.758	6	-.019	12	0	1	0	10	0	15
386			min	-1392.073	3	.403	15	-16.074	4	0	5	-.011	4	-.001	6
387		4	max	959.745	2	1.639	6	-.019	12	0	1	0	10	0	15
388			min	-1391.683	3	.375	15	-16.532	4	0	5	-.017	4	-.002	6
389		5	max	960.266	2	1.52	6	-.019	12	0	1	0	10	0	15
390			min	-1391.292	3	.347	15	-16.99	4	0	5	-.023	4	-.003	6
391		6	max	960.787	2	1.402	6	-.019	12	0	1	0	10	0	15
392			min	-1390.902	3	.319	15	-17.449	4	0	5	-.029	4	-.003	6
393		7	max	961.307	2	1.283	6	-.019	12	0	1	0	10	0	15
394			min	-1390.511	3	.291	15	-17.907	4	0	5	-.035	4	-.004	6
395		8	max	961.828	2	1.164	6	-.019	12	0	1	0	10	0	15
396			min	-1390.121	3	.263	15	-18.365	4	0	5	-.042	4	-.004	6
397		9	max	962.349	2	1.045	6	-.019	12	0	1	0	10	0	15
398			min	-1389.73	3	.235	15	-18.824	4	0	5	-.048	4	-.004	6
399		10	max	962.869	2	.945	2	-.019	12	0	1	0	10	-.001	15
400			min	-1389.34	3	.202	12	-19.282	4	0	5	-.055	4	-.005	6
401		11	max	963.39	2	.853	2	-.019	12	0	1	0	10	-.001	15
402			min	-1388.949	3	.156	12	-19.74	4	0	5	-.062	4	-.005	6
403		12	max	963.911	2	.76	2	-.019	12	0	1	0	10	-.001	15
404			min	-1388.558	3	.109	12	-20.199	4	0	5	-.069	4	-.005	6
405		13	max	964.431	2	.668	2	-.019	12	0	1	0	10	-.001	15
406			min	-1388.168	3	.063	12	-20.657	4	0	5	-.077	4	-.005	6
407		14	max	964.952	2	.575	2	-.019	12	0	1	0	12	-.001	15
408			min	-1387.777	3	-.003	3	-21.115	4	0	5	-.084	4	-.006	6
409		15	max	965.473	2	.482	2	-.019	12	0	1	0	12	-.001	15
410			min	-1387.387	3	-.073	3	-21.574	4	0	5	-.092	4	-.006	6
411		16	max	965.994	2	.39	2	-.019	12	0	1	0	12	-.001	15
412			min	-1386.996	3	-.142	3	-22.032	4	0	5	-.099	4	-.006	6
413		17	max	966.514	2	.297	2	-.019	12	0	1	0	12	-.001	15
414			min	-1386.606	3	-.212	3	-22.49	4	0	5	-.107	4	-.006	6
415		18	max	967.035	2	.204	2	-.019	12	0	1	0	12	-.001	15
416			min	-1386.215	3	-.281	3	-22.949	4	0	5	-.115	4	-.006	2
417		19	max	967.556	2	.112	2	-.019	12	0	1	0	12	-.001	12



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
418			min	-1385.825	3	-.351	3	-23.407	4	0	5	-.124	4	-.006	2
419	M11	1	max	860.209	2	7.643	6	5.009	4	0	1	0	12	.006	2
420			min	-958.858	3	1.787	15	-.181	1	0	4	-.023	4	.001	12
421		2	max	860.039	2	6.882	6	5.544	4	0	1	0	12	.004	2
422			min	-958.985	3	1.609	15	-.181	1	0	4	-.02	4	0	3
423		3	max	859.868	2	6.121	6	6.079	4	0	1	0	12	.001	2
424			min	-959.113	3	1.43	15	-.181	1	0	4	-.018	4	-.001	3
425		4	max	859.698	2	5.36	6	6.613	4	0	1	0	12	0	2
426			min	-959.241	3	1.251	15	-.181	1	0	4	-.015	4	-.003	3
427		5	max	859.528	2	4.599	6	7.148	4	0	1	0	12	-.001	15
428			min	-959.369	3	1.072	15	-.181	1	0	4	-.012	4	-.004	4
429		6	max	859.357	2	3.838	6	7.683	4	0	1	0	12	-.001	15
430			min	-959.496	3	.893	15	-.181	1	0	4	-.009	4	-.006	4
431		7	max	859.187	2	3.077	6	8.218	4	0	1	0	12	-.002	15
432			min	-959.624	3	.714	15	-.181	1	0	4	-.006	4	-.007	4
433		8	max	859.017	2	2.316	6	8.752	4	0	1	0	12	-.002	15
434			min	-959.752	3	.535	15	-.181	1	0	4	-.003	4	-.009	4
435		9	max	858.846	2	1.555	6	9.287	4	0	1	.001	5	-.002	15
436			min	-959.88	3	.356	15	-.181	1	0	4	0	1	-.009	4
437		10	max	858.676	2	.81	2	9.822	4	0	1	.005	5	-.002	15
438			min	-960.007	3	.173	12	-.181	1	0	4	0	1	-.01	4
439		11	max	858.506	2	.217	2	10.356	4	0	1	.01	5	-.002	15
440			min	-960.135	3	-.209	3	-.181	1	0	4	0	1	-.01	4
441		12	max	858.335	2	-.18	15	10.891	4	0	1	.014	5	-.002	15
442			min	-960.263	3	-.729	4	-.181	1	0	4	-.001	1	-.01	4
443		13	max	858.165	2	-.359	15	11.426	4	0	1	.019	5	-.002	15
444			min	-960.391	3	-1.49	4	-.181	1	0	4	-.001	1	-.009	4
445		14	max	857.995	2	-.538	15	11.96	4	0	1	.023	5	-.002	15
446			min	-960.518	3	-2.251	4	-.181	1	0	4	-.001	1	-.009	4
447		15	max	857.824	2	-.717	15	12.495	4	0	1	.028	5	-.002	15
448			min	-960.646	3	-3.012	4	-.181	1	0	4	-.001	1	-.008	4
449		16	max	857.654	2	-.896	15	13.03	4	0	1	.034	5	-.001	15
450			min	-960.774	3	-3.773	4	-.181	1	0	4	-.001	1	-.006	4
451		17	max	857.484	2	-1.075	15	13.564	4	0	1	.039	5	-.001	15
452			min	-960.902	3	-4.534	4	-.181	1	0	4	-.001	1	-.004	4
453		18	max	857.313	2	-1.253	15	14.099	4	0	1	.045	4	0	15
454			min	-961.03	3	-5.295	4	-.181	1	0	4	-.001	1	-.002	4
455		19	max	857.143	2	-1.432	15	14.634	4	0	1	.051	4	0	1
456			min	-961.157	3	-6.056	4	-.181	1	0	4	-.002	1	0	1
457	M12	1	max	730.655	1	0	1	6.167	1	0	1	.048	4	0	1
458			min	59.046	12	0	1	-222.803	4	0	1	-.001	1	0	1
459		2	max	730.825	1	0	1	6.167	1	0	1	.023	5	0	1
460			min	59.131	12	0	1	-222.951	4	0	1	0	1	0	1
461		3	max	730.995	1	0	1	6.167	1	0	1	0	10	0	1
462			min	59.216	12	0	1	-223.099	4	0	1	-.003	4	0	1
463		4	max	731.166	1	0	1	6.167	1	0	1	0	1	0	1
464			min	59.301	12	0	1	-223.246	4	0	1	-.028	4	0	1
465		5	max	731.336	1	0	1	6.167	1	0	1	.001	1	0	1
466			min	59.386	12	0	1	-223.394	4	0	1	-.054	4	0	1
467		6	max	731.506	1	0	1	6.167	1	0	1	.002	1	0	1
468			min	59.472	12	0	1	-223.541	4	0	1	-.08	4	0	1
469		7	max	731.677	1	0	1	6.167	1	0	1	.003	1	0	1
470			min	59.557	12	0	1	-223.689	4	0	1	-.105	4	0	1
471		8	max	731.847	1	0	1	6.167	1	0	1	.003	1	0	1
472			min	59.642	12	0	1	-223.837	4	0	1	-.131	4	0	1
473		9	max	732.017	1	0	1	6.167	1	0	1	.004	1	0	1
474			min	59.727	12	0	1	-223.984	4	0	1	-.157	4	0	1



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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
475	10	max	732.188	1	0	1	6.167	1	0	1	.005	1	0	1
476		min	59.812	12	0	1	-224.132	4	0	1	-.183	4	0	1
477	11	max	732.358	1	0	1	6.167	1	0	1	.006	1	0	1
478		min	59.897	12	0	1	-224.28	4	0	1	-.208	4	0	1
479	12	max	732.528	1	0	1	6.167	1	0	1	.006	1	0	1
480		min	59.983	12	0	1	-224.427	4	0	1	-.234	4	0	1
481	13	max	732.699	1	0	1	6.167	1	0	1	.007	1	0	1
482		min	60.068	12	0	1	-224.575	4	0	1	-.26	4	0	1
483	14	max	732.869	1	0	1	6.167	1	0	1	.008	1	0	1
484		min	60.153	12	0	1	-224.723	4	0	1	-.286	4	0	1
485	15	max	733.039	1	0	1	6.167	1	0	1	.008	1	0	1
486		min	60.238	12	0	1	-224.87	4	0	1	-.311	4	0	1
487	16	max	733.21	1	0	1	6.167	1	0	1	.009	1	0	1
488		min	60.323	12	0	1	-225.018	4	0	1	-.337	4	0	1
489	17	max	733.38	1	0	1	6.167	1	0	1	.01	1	0	1
490		min	60.408	12	0	1	-225.165	4	0	1	-.363	4	0	1
491	18	max	733.55	1	0	1	6.167	1	0	1	.011	1	0	1
492		min	60.494	12	0	1	-225.313	4	0	1	-.389	4	0	1
493	19	max	733.721	1	0	1	6.167	1	0	1	.011	1	0	1
494		min	60.579	12	0	1	-225.461	4	0	1	-.415	4	0	1
495	M1	1	max	126.58	1	714.029	3	33.315	5	0	.127	1	0	15
496		min	-14.355	5	-375.144	2	-53.239	1	0	3	-.085	5	-.013	2
497	2	max	127.402	1	713.149	3	34.557	5	0	2	.099	1	.185	2
498		min	-13.972	5	-376.317	2	-53.239	1	0	3	-.067	5	-.378	3
499	3	max	602.567	3	488.246	2	21.049	5	0	3	.07	1	.374	2
500		min	-347.044	2	-555.028	3	-53.123	1	0	2	-.049	5	-.739	3
501	4	max	603.184	3	487.073	2	22.29	5	0	3	.042	1	.117	2
502		min	-346.223	2	-555.908	3	-53.123	1	0	2	-.037	5	-.445	3
503	5	max	603.8	3	485.9	2	23.532	5	0	3	.014	1	-.003	15
504		min	-345.401	2	-556.788	3	-53.123	1	0	2	-.025	5	-.152	3
505	6	max	604.416	3	484.726	2	24.773	5	0	3	-.001	12	.142	3
506		min	-344.579	2	-557.668	3	-53.123	1	0	2	-.016	4	-.396	2
507	7	max	605.032	3	483.553	2	26.015	5	0	3	.001	5	.437	3
508		min	-343.758	2	-558.548	3	-53.123	1	0	2	-.042	1	-.652	2
509	8	max	605.649	3	482.38	2	27.256	5	0	3	.015	5	.732	3
510		min	-342.936	2	-559.428	3	-53.123	1	0	2	-.07	1	-.907	2
511	9	max	620.763	3	53.092	2	50.406	5	0	9	.044	1	.85	3
512		min	-286.534	2	.354	15	-83.497	1	0	3	-.1	5	-1.037	2
513	10	max	621.379	3	51.919	2	51.647	5	0	9	0	10	.832	3
514		min	-285.712	2	0	5	-83.497	1	0	3	-.074	4	-1.064	2
515	11	max	621.995	3	50.745	2	52.889	5	0	9	-.005	12	.815	3
516		min	-284.89	2	-1.474	4	-83.497	1	0	3	-.057	4	-1.092	2
517	12	max	636.824	3	385.118	3	125.865	5	0	2	.069	1	.714	3
518		min	-228.355	2	-594.194	2	-52.252	1	0	3	-.187	5	-.97	2
519	13	max	637.44	3	384.238	3	127.106	5	0	2	.041	1	.511	3
520		min	-227.533	2	-595.367	2	-52.252	1	0	3	-.12	5	-.656	2
521	14	max	638.057	3	383.358	3	128.348	5	0	2	.014	1	.309	3
522		min	-226.712	2	-596.54	2	-52.252	1	0	3	-.052	5	-.342	2
523	15	max	638.673	3	382.478	3	129.589	5	0	2	.016	5	.106	3
524		min	-225.89	2	-597.714	2	-52.252	1	0	3	-.014	1	-.037	1
525	16	max	639.289	3	381.598	3	130.831	5	0	2	.084	5	.289	2
526		min	-225.069	2	-598.887	2	-52.252	1	0	3	-.041	1	-.095	3
527	17	max	639.905	3	380.718	3	132.072	5	0	2	.154	5	.605	2
528		min	-224.247	2	-600.061	2	-52.252	1	0	3	-.069	1	-.296	3
529	18	max	27.695	5	594.335	2	-5.663	12	0	5	.176	5	.306	2
530		min	-127.802	1	-299.571	3	-90.888	4	0	2	-.098	1	-.147	3
531	19	max	28.078	5	593.161	2	-5.663	12	0	5	.137	5	.012	3





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
532		min	-126.98	1	-300.451	3	-89.646	4	0	2	-.128	1	-.007	2
533	M5	max	284.077	1	2365.066	3	72.88	5	0	1	0	1	.026	2
534		min	11.455	12	-1296.878	2	0	1	0	4	-.176	4	0	15
535		max	284.899	1	2364.186	3	74.122	5	0	1	0	1	.71	2
536		min	11.865	12	-1298.051	2	0	1	0	4	-.138	4	-1.245	3
537		max	1866.633	3	1348.725	2	61.901	4	0	4	0	1	1.364	2
538		min	-1111.612	2	-1656.072	3	0	1	0	1	-.098	4	-2.444	3
539		max	1867.249	3	1347.551	2	63.143	4	0	4	0	1	.652	2
540		min	-1110.791	2	-1656.952	3	0	1	0	1	-.065	4	-1.57	3
541		max	1867.866	3	1346.378	2	64.384	4	0	4	0	1	.014	9
542		min	-1109.969	2	-1657.832	3	0	1	0	1	-.032	4	-.696	3
543		max	1868.482	3	1345.205	2	65.626	4	0	4	.003	4	.179	3
544		min	-1109.148	2	-1658.712	3	0	1	0	1	0	1	-.768	2
545		max	1869.098	3	1344.031	2	66.867	4	0	4	.038	4	1.055	3
546		min	-1108.326	2	-1659.592	3	0	1	0	1	0	1	-1.478	2
547		max	1869.714	3	1342.858	2	68.108	4	0	4	.073	4	1.931	3
548		min	-1107.504	2	-1660.472	3	0	1	0	1	0	1	-2.187	2
549		max	1885.902	3	179.834	2	169.619	4	0	1	0	1	2.218	3
550		min	-982.235	2	.35	15	0	1	0	1	-.156	4	-2.503	2
551		max	1886.518	3	178.66	2	170.861	4	0	1	0	1	2.151	3
552		min	-981.413	2	-.004	15	0	1	0	1	-.066	4	-2.598	2
553		max	1887.134	3	177.487	2	172.102	4	0	1	.025	4	2.084	3
554		min	-980.591	2	-1.423	6	0	1	0	1	0	1	-2.692	2
555		max	1903.893	3	1114.339	3	184.194	4	0	1	0	1	1.829	3
556		min	-855.588	2	-1705.122	2	0	1	0	4	-.27	4	-2.414	2
557		max	1904.509	3	1113.459	3	185.435	4	0	1	0	1	1.242	3
558		min	-854.766	2	-1706.295	2	0	1	0	4	-.173	4	-1.514	2
559		max	1905.125	3	1112.579	3	186.677	4	0	1	0	1	.654	3
560		min	-853.944	2	-1707.469	2	0	1	0	4	-.075	4	-.613	2
561		max	1905.742	3	1111.699	3	187.918	4	0	1	.024	4	.288	2
562		min	-853.123	2	-1708.642	2	0	1	0	4	0	1	0	13
563		max	1906.358	3	1110.819	3	189.159	4	0	1	.124	4	1.19	2
564		min	-852.301	2	-1709.815	2	0	1	0	4	0	1	-.519	3
565		max	1906.974	3	1109.939	3	190.401	4	0	1	.224	4	2.092	2
566		min	-851.48	2	-1710.989	2	0	1	0	4	0	1	-1.105	3
567		max	-13.005	12	2011.11	2	0	1	0	4	.278	4	1.076	2
568		min	-284.099	1	-1049.28	3	-12.152	5	0	1	0	1	-.577	3
569		max	-12.595	12	2009.937	2	0	1	0	4	.272	4	.015	2
570		min	-283.277	1	-1050.16	3	-10.91	5	0	1	0	1	-.023	3
571	M9	max	126.58	1	714.029	3	56.791	4	0	3	-.014	12	0	15
572		min	9.129	12	-375.144	2	5.946	12	0	4	-.141	4	-.013	2
573		max	127.402	1	713.149	3	58.032	4	0	3	-.011	12	.185	2
574		min	9.539	12	-376.317	2	5.946	12	0	4	-.111	4	-.378	3
575		max	602.567	3	488.246	2	53.123	1	0	2	-.008	12	.374	2
576		min	-347.044	2	-555.028	3	5.928	12	0	3	-.08	4	-.739	3
577		max	603.184	3	487.073	2	53.123	1	0	2	-.005	10	.117	2
578		min	-346.223	2	-555.908	3	5.928	12	0	3	-.056	4	-.445	3
579		max	603.8	3	485.9	2	53.123	1	0	2	-.002	10	-.003	15
580		min	-345.401	2	-556.788	3	5.928	12	0	3	-.032	4	-.152	3
581		max	604.416	3	484.726	2	53.123	1	0	2	.014	1	.142	3
582		min	-344.579	2	-557.668	3	5.928	12	0	3	-.01	5	-.396	2
583		max	605.032	3	483.553	2	53.123	1	0	2	.042	1	.437	3
584		min	-343.758	2	-558.548	3	5.928	12	0	3	.005	12	-.652	2
585		max	605.649	3	482.38	2	53.123	1	0	2	.07	1	.732	3
586		min	-342.936	2	-559.428	3	5.928	12	0	3	.008	12	-.907	2
587		max	620.763	3	53.092	2	87.31	4	0	3	-.004	12	.85	3
588		min	-286.534	2	.363	15	8.711	12	0	9	-.12	4	-1.037	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
589	10	max	621.379	3	51.919	2	88.551	4	0	3	0	1	.832	3
590		min	-285.712	2	.009	15	8.711	12	0	9	-.073	4	-1.064	2
591	11	max	621.995	3	50.745	2	89.793	4	0	3	.044	1	.815	3
592		min	-284.89	2	-1.423	6	8.711	12	0	9	-.038	5	-1.092	2
593	12	max	636.824	3	385.118	3	149.645	4	0	3	-.007	12	.714	3
594		min	-228.355	2	-594.194	2	5.098	12	0	2	-.218	4	-.97	2
595	13	max	637.44	3	384.238	3	150.886	4	0	3	-.004	12	.511	3
596		min	-227.533	2	-595.367	2	5.098	12	0	2	-.139	4	-.656	2
597	14	max	638.057	3	383.358	3	152.128	4	0	3	-.001	12	.309	3
598		min	-226.712	2	-596.54	2	5.098	12	0	2	-.059	4	-.342	2
599	15	max	638.673	3	382.478	3	153.369	4	0	3	.022	4	.106	3
600		min	-225.89	2	-597.714	2	5.098	12	0	2	.001	12	-.037	1
601	16	max	639.289	3	381.598	3	154.611	4	0	3	.103	4	.289	2
602		min	-225.069	2	-598.887	2	5.098	12	0	2	.004	12	-.095	3
603	17	max	639.905	3	380.718	3	155.852	4	0	3	.185	4	.605	2
604		min	-224.247	2	-600.061	2	5.098	12	0	2	.007	12	-.296	3
605	18	max	-8.969	12	594.335	2	58.14	1	0	2	.22	4	.306	2
606		min	-127.802	1	-299.571	3	-65.209	5	0	3	.009	12	-.147	3
607	19	max	-8.559	12	593.161	2	58.14	1	0	2	.194	4	.012	3
608		min	-126.98	1	-300.451	3	-63.967	5	0	3	.012	12	-.007	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.112	2	.01	3	9.493e-3	2	NC	1	NC	1
2				min	-466	4	-.03	3	-.006	2	-2.991e-3	3	NC	1	NC
3		2	max	0	1	.103	3	.012	3	1.041e-2	2	NC	4	NC	1
4			min	-466	4	.002	15	-.009	5	-2.905e-3	3	1313.353	3	NC	1
5		3	max	0	1	.211	3	.027	1	1.132e-2	2	NC	4	NC	2
6			min	-466	4	-.003	9	-.011	5	-2.819e-3	3	723.647	3	6234.536	1
7		4	max	0	1	.278	3	.04	1	1.223e-2	2	NC	4	NC	2
8			min	-466	4	-.012	1	-.008	5	-2.733e-3	3	566.072	3	4238.165	1
9		5	max	0	1	.296	3	.046	1	1.314e-2	2	NC	4	NC	2
10			min	-466	4	-.009	1	-.003	5	-2.647e-3	3	534.535	3	3697.443	1
11		6	max	0	1	.266	3	.043	1	1.406e-2	2	NC	4	NC	2
12			min	-466	4	-.002	9	-.002	10	-2.561e-3	3	588.178	3	3947.539	1
13		7	max	0	1	.198	3	.032	1	1.497e-2	2	NC	4	NC	2
14			min	-466	4	.001	15	-.005	10	-2.475e-3	3	763.04	3	5312.044	1
15		8	max	0	1	.134	2	.03	3	1.588e-2	2	NC	1	NC	1
16			min	-466	4	.002	15	-.01	2	-2.389e-3	3	1237.605	3	8710.727	3
17		9	max	0	1	.182	2	.031	3	1.679e-2	2	NC	4	NC	1
18			min	-466	4	.003	15	-.018	2	-2.303e-3	3	2479.793	2	8527.698	3
19		10	max	0	1	.204	2	.031	3	1.771e-2	2	NC	4	NC	1
20			min	-466	4	-.006	3	-.022	2	-2.218e-3	3	1896.897	2	8515.845	3
21		11	max	0	12	.182	2	.031	3	1.679e-2	2	NC	4	NC	1
22			min	-466	4	.003	15	-.018	2	-2.303e-3	3	2479.793	2	8527.698	3
23		12	max	0	12	.134	2	.03	3	1.588e-2	2	NC	1	NC	1
24			min	-466	4	.002	15	-.01	2	-2.389e-3	3	1237.605	3	8710.727	3
25		13	max	0	12	.198	3	.032	1	1.497e-2	2	NC	4	NC	2
26			min	-466	4	.001	15	-.005	10	-2.475e-3	3	763.04	3	5312.044	1
27		14	max	0	12	.266	3	.043	1	1.406e-2	2	NC	4	NC	2
28			min	-466	4	-.002	9	-.002	10	-2.561e-3	3	588.178	3	3947.539	1
29		15	max	0	12	.296	3	.046	1	1.314e-2	2	NC	4	NC	2
30			min	-466	4	-.009	1	0	10	-2.647e-3	3	534.535	3	3697.443	1
31		16	max	0	12	.278	3	.04	1	1.223e-2	2	NC	4	NC	2
32			min	-466	4	-.012	1	0	10	-2.733e-3	3	566.072	3	4238.165	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
33	17	max	0	12	.211	3	.027	1	1.132e-2	2	NC	4	NC	2
34		min	-466	4	-.003	9	-.001	10	-2.819e-3	3	723.647	3	6234.536	1
35	18	max	0	12	.103	3	.016	4	1.041e-2	2	NC	4	NC	1
36		min	-466	4	0	15	-.003	10	-2.905e-3	3	1313.353	3	NC	1
37	19	max	0	12	.112	2	.01	3	9.493e-3	2	NC	1	NC	1
38		min	-466	4	-.03	3	-.006	2	-2.991e-3	3	NC	1	NC	1
39	M14	1	max	0	.264	3	.009	3	5.237e-3	2	NC	1	NC	1
40		min	-.355	4	-.354	2	-.006	2	-4.449e-3	3	NC	1	NC	1
41	2	max	0	1	.429	3	.01	3	6.071e-3	2	NC	4	NC	1
42		min	-.355	4	-.502	2	-.014	5	-5.23e-3	3	1057.235	3	NC	1
43	3	max	0	1	.573	3	.021	1	6.906e-3	2	NC	5	NC	2
44		min	-.355	4	-.634	2	-.018	5	-6.012e-3	3	563.757	3	8164.517	1
45	4	max	0	1	.682	3	.033	1	7.74e-3	2	NC	5	NC	2
46		min	-.355	4	-.741	2	-.013	5	-6.794e-3	3	416.393	3	5136.292	1
47	5	max	0	1	.749	3	.04	1	8.574e-3	2	NC	5	NC	2
48		min	-.355	4	-.816	2	-.003	5	-7.576e-3	3	358.966	3	4291.234	1
49	6	max	0	1	.773	3	.038	1	9.409e-3	2	NC	5	NC	2
50		min	-.355	4	-.858	2	-.002	10	-8.358e-3	3	342.239	3	4454.655	1
51	7	max	0	1	.759	3	.029	1	1.024e-2	2	NC	5	NC	2
52		min	-.355	4	-.869	2	-.005	10	-9.14e-3	3	337.619	2	5871.099	1
53	8	max	0	1	.721	3	.027	4	1.108e-2	2	NC	5	NC	1
54		min	-.355	4	-.858	2	-.009	2	-9.922e-3	3	344.715	2	6662.203	4
55	9	max	0	1	.679	3	.027	3	1.191e-2	2	NC	5	NC	1
56		min	-.355	4	-.839	2	-.017	2	-1.07e-2	3	358.764	2	9615.299	3
57	10	max	0	1	.657	3	.027	3	1.275e-2	2	NC	5	NC	1
58		min	-.355	4	-.827	2	-.02	2	-1.149e-2	3	367.388	2	9587.275	3
59	11	max	0	12	.679	3	.027	3	1.191e-2	2	NC	5	NC	1
60		min	-.355	4	-.839	2	-.017	2	-1.07e-2	3	358.764	2	9615.299	3
61	12	max	0	12	.721	3	.027	3	1.108e-2	2	NC	5	NC	1
62		min	-.355	4	-.858	2	-.017	5	-9.922e-3	3	344.715	2	9874.153	3
63	13	max	0	12	.759	3	.029	1	1.024e-2	2	NC	5	NC	2
64		min	-.355	4	-.869	2	-.011	5	-9.14e-3	3	337.619	2	5871.099	1
65	14	max	0	12	.773	3	.038	1	9.409e-3	2	NC	5	NC	2
66		min	-.355	4	-.858	2	-.002	10	-8.358e-3	3	342.239	3	4454.655	1
67	15	max	0	12	.749	3	.04	1	8.574e-3	2	NC	5	NC	2
68		min	-.355	4	-.816	2	0	10	-7.576e-3	3	358.966	3	4291.234	1
69	16	max	0	12	.682	3	.033	1	7.74e-3	2	NC	5	NC	2
70		min	-.355	4	-.741	2	0	10	-6.794e-3	3	416.393	3	5136.292	1
71	17	max	0	12	.573	3	.029	4	6.906e-3	2	NC	5	NC	2
72		min	-.355	4	-.634	2	-.001	10	-6.012e-3	3	563.757	3	6033.895	4
73	18	max	0	12	.429	3	.019	4	6.071e-3	2	NC	4	NC	1
74		min	-.355	4	-.502	2	-.003	2	-5.23e-3	3	1057.235	3	8948	4
75	19	max	0	12	.264	3	.009	3	5.237e-3	2	NC	1	NC	1
76		min	-.355	4	-.354	2	-.006	2	-4.449e-3	3	NC	1	NC	1
77	M15	1	max	0	.268	3	.008	3	3.958e-3	3	NC	1	NC	1
78		min	-.294	4	-.352	2	-.005	2	-5.512e-3	2	NC	1	NC	1
79	2	max	0	10	.39	3	.01	3	4.656e-3	3	NC	4	NC	1
80		min	-.294	4	-.539	2	-.02	5	-6.399e-3	2	932.548	2	8170.981	5
81	3	max	0	10	.5	3	.021	1	5.354e-3	3	NC	5	NC	2
82		min	-.294	4	-.703	2	-.025	5	-7.285e-3	2	496.195	2	6673.876	5
83	4	max	0	10	.588	3	.033	1	6.052e-3	3	NC	5	NC	2
84		min	-.294	4	-.829	2	-.018	5	-8.171e-3	2	365.169	2	5112.468	1
85	5	max	0	10	.651	3	.04	1	6.75e-3	3	NC	5	NC	2
86		min	-.294	4	-.908	2	-.005	5	-9.058e-3	2	313.143	2	4268.878	1
87	6	max	0	10	.686	3	.039	1	7.448e-3	3	NC	5	NC	2
88		min	-.294	4	-.94	2	-.002	10	-9.944e-3	2	296.358	2	4424.846	1
89	7	max	0	10	.697	3	.031	4	8.146e-3	3	NC	5	NC	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
90		min	-.294	4	-.93	2	-.004	10	-1.083e-2	2	301.441	2	5810.811	1
91	8	max	0	10	.69	3	.033	4	8.844e-3	3	NC	5	NC	1
92		min	-.294	4	-.892	2	-.008	2	-1.172e-2	2	322.441	2	5478.62	4
93	9	max	0	10	.675	3	.025	3	9.542e-3	3	NC	5	NC	1
94		min	-.294	4	-.848	2	-.015	2	-1.26e-2	2	351.237	2	7723.586	4
95	10	max	0	1	.666	3	.025	3	1.024e-2	3	NC	5	NC	1
96		min	-.294	4	-.825	2	-.019	2	-1.349e-2	2	367.944	2	NC	1
97	11	max	0	1	.675	3	.025	3	9.542e-3	3	NC	5	NC	1
98		min	-.294	4	-.848	2	-.019	5	-1.26e-2	2	351.237	2	9022.811	5
99	12	max	0	1	.69	3	.025	3	8.844e-3	3	NC	5	NC	1
100		min	-.294	4	-.892	2	-.023	5	-1.172e-2	2	322.441	2	7718.291	5
101	13	max	0	1	.697	3	.029	1	8.146e-3	3	NC	5	NC	2
102		min	-.294	4	-.93	2	-.015	5	-1.083e-2	2	301.441	2	5810.811	1
103	14	max	0	1	.686	3	.039	1	7.448e-3	3	NC	5	NC	2
104		min	-.294	4	-.94	2	-.002	10	-9.944e-3	2	296.358	2	4424.846	1
105	15	max	0	1	.651	3	.04	1	6.75e-3	3	NC	5	NC	2
106		min	-.294	4	-.908	2	0	10	-9.058e-3	2	313.143	2	4268.878	1
107	16	max	0	1	.588	3	.035	4	6.052e-3	3	NC	5	NC	2
108		min	-.294	4	-.829	2	0	10	-8.171e-3	2	365.169	2	4997.371	4
109	17	max	0	1	.5	3	.036	4	5.354e-3	3	NC	5	NC	2
110		min	-.294	4	-.703	2	-.001	10	-7.285e-3	2	496.195	2	4865.465	4
111	18	max	0	1	.39	3	.024	4	4.656e-3	3	NC	4	NC	1
112		min	-.293	4	-.539	2	-.003	10	-6.399e-3	2	932.548	2	7005.482	4
113	19	max	0	1	.268	3	.008	3	3.958e-3	3	NC	1	NC	1
114		min	-.293	4	-.352	2	-.005	2	-5.512e-3	2	NC	1	NC	1
115	M16	1	max	0	.099	2	.007	3	7.263e-3	3	NC	1	NC	1
116		min	-.109	4	-.089	3	-.005	2	-7.778e-3	2	NC	1	NC	1
117	2	max	0	12	.012	1	.011	1	8.095e-3	3	NC	4	NC	1
118		min	-.109	4	-.049	3	-.016	5	-8.305e-3	2	1786.746	2	NC	1
119	3	max	0	12	.002	4	.027	1	8.926e-3	3	NC	4	NC	2
120		min	-.109	4	-.076	2	-.02	5	-8.832e-3	2	997.286	2	6214.387	1
121	4	max	0	12	0	5	.041	1	9.758e-3	3	NC	4	NC	2
122		min	-.109	4	-.119	2	-.016	5	-9.359e-3	2	799.186	2	4207.765	1
123	5	max	0	12	0	13	.047	1	1.059e-2	3	NC	4	NC	2
124		min	-.109	4	-.122	2	-.007	5	-9.887e-3	2	787.547	2	3653.608	1
125	6	max	0	12	.003	4	.044	1	1.142e-2	3	NC	4	NC	2
126		min	-.109	4	-.087	2	0	10	-1.041e-2	2	939.028	2	3871.961	1
127	7	max	0	12	.013	9	.033	1	1.225e-2	3	NC	3	NC	2
128		min	-.109	4	-.079	3	-.003	10	-1.094e-2	2	1458.16	2	5131.888	1
129	8	max	0	12	.061	1	.023	4	1.308e-2	3	NC	1	NC	2
130		min	-.109	4	-.125	3	-.006	10	-1.147e-2	2	4470.565	2	7860.893	4
131	9	max	0	12	.131	2	.022	3	1.392e-2	3	NC	4	NC	1
132		min	-.109	4	-.166	3	-.014	2	-1.2e-2	2	2280.581	3	NC	1
133	10	max	0	1	.164	2	.022	3	1.475e-2	3	NC	4	NC	1
134		min	-.109	4	-.183	3	-.017	2	-1.252e-2	2	1850.785	3	NC	1
135	11	max	0	1	.131	2	.022	3	1.392e-2	3	NC	4	NC	1
136		min	-.109	4	-.166	3	-.014	2	-1.2e-2	2	2280.581	3	NC	1
137	12	max	0	1	.061	1	.022	3	1.308e-2	3	NC	1	NC	2
138		min	-.109	4	-.125	3	-.012	5	-1.147e-2	2	4470.565	2	9872.911	1
139	13	max	0	1	.013	9	.033	1	1.225e-2	3	NC	3	NC	2
140		min	-.109	4	-.079	3	-.005	5	-1.094e-2	2	1458.16	2	5131.888	1
141	14	max	0	1	.003	6	.044	1	1.142e-2	3	NC	4	NC	2
142		min	-.109	4	-.087	2	0	10	-1.041e-2	2	939.028	2	3871.961	1
143	15	max	0	1	0	13	.047	1	1.059e-2	3	NC	4	NC	2
144		min	-.109	4	-.122	2	.001	10	-9.887e-3	2	787.547	2	3653.608	1
145	16	max	0	1	0	13	.041	1	9.758e-3	3	NC	4	NC	2
146		min	-.109	4	-.119	2	.001	10	-9.359e-3	2	799.186	2	4207.765	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
147	17	max	0	1	.001	13	.034	4	8.926e-3	3	NC	4	NC	2
148		min	-.109	4	-.076	2	0	10	-8.832e-3	2	997.286	2	5156.631	4
149	18	max	0	1	.012	1	.022	4	8.095e-3	3	NC	4	NC	1
150		min	-.109	4	-.049	3	-.002	10	-8.305e-3	2	1786.746	2	7833.646	4
151	19	max	0	1	.099	2	.007	3	7.263e-3	3	NC	1	NC	1
152		min	-.109	4	-.089	3	-.005	2	-7.778e-3	2	NC	1	NC	1
153	M2	1	max	.007	2	.01	.004	1	1.334e-3	5	NC	1	NC	1
154		min	-.01	3	-.016	3	-.439	4	-1.124e-4	1	7425.377	2	175.289	4
155	2	max	.007	2	.009	2	.004	1	1.37e-3	5	NC	1	NC	1
156		min	-.01	3	-.015	3	-.404	4	-1.066e-4	1	8607.117	2	190.489	4
157	3	max	.006	2	.008	2	.003	1	1.406e-3	5	NC	1	NC	1
158		min	-.009	3	-.015	3	-.369	4	-1.008e-4	1	NC	1	208.477	4
159	4	max	.006	2	.006	2	.003	1	1.441e-3	5	NC	1	NC	1
160		min	-.009	3	-.014	3	-.335	4	-9.498e-5	1	NC	1	229.969	4
161	5	max	.006	2	.005	2	.003	1	1.477e-3	5	NC	1	NC	1
162		min	-.008	3	-.014	3	-.301	4	-8.919e-5	1	NC	1	255.936	4
163	6	max	.005	2	.004	2	.002	1	1.513e-3	5	NC	1	NC	1
164		min	-.007	3	-.013	3	-.268	4	-8.339e-5	1	NC	1	287.718	4
165	7	max	.005	2	.002	2	.002	1	1.549e-3	5	NC	1	NC	1
166		min	-.007	3	-.013	3	-.235	4	-7.76e-5	1	NC	1	327.206	4
167	8	max	.004	2	.001	2	.002	1	1.584e-3	5	NC	1	NC	1
168		min	-.006	3	-.012	3	-.204	4	-7.181e-5	1	NC	1	377.141	4
169	9	max	.004	2	0	2	.002	1	1.62e-3	5	NC	1	NC	1
170		min	-.006	3	-.011	3	-.174	4	-6.602e-5	1	NC	1	441.623	4
171	10	max	.004	2	0	2	.001	1	1.656e-3	5	NC	1	NC	1
172		min	-.005	3	-.01	3	-.146	4	-6.023e-5	1	NC	1	527.009	4
173	11	max	.003	2	0	2	.001	1	1.692e-3	5	NC	1	NC	1
174		min	-.005	3	-.009	3	-.12	4	-5.444e-5	1	NC	1	643.607	4
175	12	max	.003	2	-.001	15	0	1	1.727e-3	5	NC	1	NC	1
176		min	-.004	3	-.008	3	-.095	4	-4.865e-5	1	NC	1	809.049	4
177	13	max	.002	2	-.001	15	0	1	1.764e-3	4	NC	1	NC	1
178		min	-.003	3	-.007	3	-.073	4	-4.286e-5	1	NC	1	1055.622	4
179	14	max	.002	2	0	15	0	1	1.801e-3	4	NC	1	NC	1
180		min	-.003	3	-.006	3	-.053	4	-3.707e-5	1	NC	1	1447.882	4
181	15	max	.002	2	0	15	0	1	1.839e-3	4	NC	1	NC	1
182		min	-.002	3	-.005	3	-.036	4	-3.127e-5	1	NC	1	2131.274	4
183	16	max	.001	2	0	15	0	1	1.876e-3	4	NC	1	NC	1
184		min	-.002	3	-.004	3	-.022	4	-2.548e-5	1	NC	1	3495.106	4
185	17	max	0	2	0	15	0	1	1.914e-3	4	NC	1	NC	1
186		min	-.001	3	-.003	3	-.011	4	-1.969e-5	1	NC	1	6912.974	4
187	18	max	0	2	0	15	0	1	1.951e-3	4	NC	1	NC	1
188		min	0	3	-.001	3	-.004	4	-1.39e-5	1	NC	1	NC	1
189	19	max	0	1	0	1	0	1	1.989e-3	4	NC	1	NC	1
190		min	0	1	0	1	0	1	-8.11e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	0	1	1.837e-6	1	NC	1	NC	1
192		min	0	1	0	1	0	1	-5.108e-4	4	NC	1	NC	1
193	2	max	0	3	0	15	.009	4	1.235e-5	1	NC	1	NC	1
194		min	0	2	-.002	6	0	1	-5.57e-5	5	NC	1	9646.074	4
195	3	max	0	3	0	15	.018	4	4.068e-4	4	NC	1	NC	1
196		min	0	2	-.004	6	0	1	2.2e-6	12	NC	1	5064.834	4
197	4	max	.001	3	-.001	15	.025	4	8.657e-4	4	NC	1	NC	1
198		min	-.001	2	-.006	6	0	1	3.157e-6	12	NC	1	3539.496	4
199	5	max	.002	3	-.002	15	.032	4	1.325e-3	4	NC	1	NC	1
200		min	-.002	2	-.008	6	0	1	4.114e-6	12	NC	1	2775.842	4
201	6	max	.002	3	-.002	15	.039	4	1.783e-3	4	NC	1	NC	1
202		min	-.002	2	-.01	6	0	1	5.071e-6	12	9376.446	6	2314.56	4
203	7	max	.003	3	-.002	15	.045	4	2.242e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
204			min	-.002	2	-.011	6	0	3	6.029e-6	12	8100.454	6	2002.173	4
205		8	max	.003	3	-.003	15	.051	4	2.701e-3	4	NC	2	NC	1
206			min	-.003	2	-.012	6	0	12	6.986e-6	12	7314.88	6	1772.635	4
207		9	max	.004	3	-.003	15	.056	4	3.16e-3	4	NC	5	NC	1
208			min	-.003	2	-.013	6	0	12	7.943e-6	12	6855.702	6	1592.784	4
209		10	max	.004	3	-.003	15	.062	4	3.619e-3	4	NC	5	NC	1
210			min	-.004	2	-.014	6	0	12	8.9e-6	12	6643.859	6	1444.197	4
211		11	max	.005	3	-.003	15	.068	4	4.078e-3	4	NC	5	NC	1
212			min	-.004	2	-.014	6	0	12	9.857e-6	12	6648.399	6	1315.971	4
213		12	max	.005	3	-.003	15	.075	4	4.536e-3	4	NC	5	NC	1
214			min	-.005	2	-.013	6	0	12	1.081e-5	12	6874.606	6	1201.45	4
215		13	max	.006	3	-.003	15	.082	4	4.995e-3	4	NC	2	NC	1
216			min	-.005	2	-.012	6	0	12	1.177e-5	12	7367.57	6	1096.587	4
217		14	max	.006	3	-.002	15	.09	4	5.454e-3	4	NC	1	NC	1
218			min	-.005	2	-.011	6	0	12	1.273e-5	12	8235.075	6	999.02	4
219		15	max	.006	3	-.002	15	.099	4	5.913e-3	4	NC	1	NC	1
220			min	-.006	2	-.009	6	0	12	1.369e-5	12	9714.163	6	907.501	4
221		16	max	.007	3	-.001	15	.109	4	6.372e-3	4	NC	1	NC	1
222			min	-.006	2	-.007	6	0	12	1.464e-5	12	NC	1	821.502	4
223		17	max	.007	3	0	15	.121	4	6.831e-3	4	NC	1	NC	1
224			min	-.007	2	-.006	3	0	12	1.56e-5	12	NC	1	740.914	4
225		18	max	.008	3	0	15	.135	4	7.29e-3	4	NC	1	NC	1
226			min	-.007	2	-.004	3	0	12	1.656e-5	12	NC	1	665.833	4
227		19	max	.008	3	0	2	.151	4	7.748e-3	4	NC	1	NC	1
228			min	-.007	2	-.003	3	0	12	1.751e-5	12	NC	1	596.405	4
229	M4	1	max	.002	1	.007	2	0	12	1.101e-3	4	NC	1	NC	2
230			min	0	5	-.009	3	-.151	4	7.647e-6	12	NC	1	164.673	4
231		2	max	.002	1	.007	2	0	12	1.101e-3	4	NC	1	NC	2
232			min	0	5	-.008	3	-.139	4	7.647e-6	12	NC	1	178.534	4
233		3	max	.002	1	.006	2	0	12	1.101e-3	4	NC	1	NC	2
234			min	0	5	-.008	3	-.127	4	7.647e-6	12	NC	1	195.063	4
235		4	max	.001	1	.006	2	0	12	1.101e-3	4	NC	1	NC	2
236			min	0	5	-.007	3	-.115	4	7.647e-6	12	NC	1	214.948	4
237		5	max	.001	1	.006	2	0	12	1.101e-3	4	NC	1	NC	2
238			min	0	5	-.007	3	-.104	4	7.647e-6	12	NC	1	239.123	4
239		6	max	.001	1	.005	2	0	12	1.101e-3	4	NC	1	NC	1
240			min	0	5	-.006	3	-.092	4	7.647e-6	12	NC	1	268.883	4
241		7	max	.001	1	.005	2	0	12	1.101e-3	4	NC	1	NC	1
242			min	0	5	-.006	3	-.081	4	7.647e-6	12	NC	1	306.068	4
243		8	max	.001	1	.004	2	0	12	1.101e-3	4	NC	1	NC	1
244			min	0	5	-.005	3	-.07	4	7.647e-6	12	NC	1	353.36	4
245		9	max	0	1	.004	2	0	12	1.101e-3	4	NC	1	NC	1
246			min	0	5	-.005	3	-.06	4	7.647e-6	12	NC	1	414.795	4
247		10	max	0	1	.004	2	0	12	1.101e-3	4	NC	1	NC	1
248			min	0	5	-.004	3	-.05	4	7.647e-6	12	NC	1	496.681	4
249		11	max	0	1	.003	2	0	12	1.101e-3	4	NC	1	NC	1
250			min	0	5	-.004	3	-.041	4	7.647e-6	12	NC	1	609.331	4
251		12	max	0	1	.003	2	0	12	1.101e-3	4	NC	1	NC	1
252			min	0	5	-.003	3	-.032	4	7.647e-6	12	NC	1	770.577	4
253		13	max	0	1	.002	2	0	12	1.101e-3	4	NC	1	NC	1
254			min	0	5	-.003	3	-.024	4	7.647e-6	12	NC	1	1013.499	4
255		14	max	0	1	.002	2	0	12	1.101e-3	4	NC	1	NC	1
256			min	0	5	-.002	3	-.018	4	7.647e-6	12	NC	1	1405.349	4
257		15	max	0	1	.002	2	0	12	1.101e-3	4	NC	1	NC	1
258			min	0	5	-.002	3	-.012	4	7.647e-6	12	NC	1	2101.031	4
259		16	max	0	1	.001	2	0	12	1.101e-3	4	NC	1	NC	1
260			min	0	5	-.001	3	-.007	4	7.647e-6	12	NC	1	3528.325	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
261		17	max	0	1	0	2	0	12	1.101e-3	4	NC	1	NC	1
262			min	0	5	0	3	-.003	4	7.647e-6	12	NC	1	7271.052	4
263		18	max	0	1	0	2	0	12	1.101e-3	4	NC	1	NC	1
264			min	0	5	0	3	-.001	4	7.647e-6	12	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.101e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	7.647e-6	12	NC	1	NC	1
267	M6	1	max	.022	2	.034	2	0	1	1.392e-3	4	NC	4	NC	1
268			min	-.033	3	-.049	3	-.443	4	0	1	1576.774	3	173.742	4
269		2	max	.021	2	.031	2	0	1	1.426e-3	4	NC	4	NC	1
270			min	-.031	3	-.046	3	-.408	4	0	1	1669.499	3	188.81	4
271		3	max	.019	2	.028	2	0	1	1.461e-3	4	NC	4	NC	1
272			min	-.029	3	-.043	3	-.373	4	0	1	1773.942	3	206.641	4
273		4	max	.018	2	.025	2	0	1	1.495e-3	4	NC	4	NC	1
274			min	-.027	3	-.041	3	-.338	4	0	1	1892.584	3	227.946	4
275		5	max	.017	2	.022	2	0	1	1.53e-3	4	NC	4	NC	1
276			min	-.025	3	-.038	3	-.304	4	0	1	2028.621	3	253.688	4
277		6	max	.016	2	.02	2	0	1	1.564e-3	4	NC	4	NC	1
278			min	-.023	3	-.035	3	-.27	4	0	1	2186.229	3	285.194	4
279		7	max	.015	2	.017	2	0	1	1.599e-3	4	NC	1	NC	1
280			min	-.022	3	-.032	3	-.237	4	0	1	2370.98	3	324.339	4
281		8	max	.013	2	.014	2	0	1	1.633e-3	4	NC	1	NC	1
282			min	-.02	3	-.03	3	-.206	4	0	1	2590.476	3	373.84	4
283		9	max	.012	2	.012	2	0	1	1.667e-3	4	NC	1	NC	1
284			min	-.018	3	-.027	3	-.176	4	0	1	2855.365	3	437.761	4
285		10	max	.011	2	.01	2	0	1	1.702e-3	4	NC	1	NC	1
286			min	-.016	3	-.024	3	-.147	4	0	1	3181.034	3	522.403	4
287		11	max	.01	2	.008	2	0	1	1.736e-3	4	NC	1	NC	1
288			min	-.014	3	-.021	3	-.121	4	0	1	3590.581	3	637.982	4
289		12	max	.009	2	.006	2	0	1	1.771e-3	4	NC	1	NC	1
290			min	-.013	3	-.019	3	-.096	4	0	1	4120.319	3	801.973	4
291		13	max	.007	2	.004	2	0	1	1.805e-3	4	NC	1	NC	1
292			min	-.011	3	-.016	3	-.074	4	0	1	4830.799	3	1046.373	4
293		14	max	.006	2	.003	2	0	1	1.84e-3	4	NC	1	NC	1
294			min	-.009	3	-.013	3	-.054	4	0	1	5831.054	3	1435.154	4
295		15	max	.005	2	.002	2	0	1	1.874e-3	4	NC	1	NC	1
296			min	-.007	3	-.01	3	-.036	4	0	1	7339.219	3	2112.424	4
297		16	max	.004	2	0	2	0	1	1.909e-3	4	NC	1	NC	1
298			min	-.005	3	-.008	3	-.022	4	0	1	9864.387	3	3463.855	4
299		17	max	.002	2	0	2	0	1	1.943e-3	4	NC	1	NC	1
300			min	-.004	3	-.005	3	-.011	4	0	1	NC	1	6849.88	4
301		18	max	.001	2	0	2	0	1	1.977e-3	4	NC	1	NC	1
302			min	-.002	3	-.003	3	-.004	4	0	1	NC	1	NC	1
303		19	max	0	1	0	1	0	1	2.012e-3	4	NC	1	NC	1
304			min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	0	1	NC	1	NC	1
306			min	0	1	0	1	0	1	-5.169e-4	4	NC	1	NC	1
307		2	max	.001	3	0	2	.009	4	0	1	NC	1	NC	1
308			min	-.001	2	-.003	3	0	1	-6.991e-5	5	NC	1	9534.913	4
309		3	max	.003	3	0	2	.018	4	3.778e-4	4	NC	1	NC	1
310			min	-.003	2	-.006	3	0	1	0	1	NC	1	5007.434	4
311		4	max	.004	3	-.001	15	.026	4	8.252e-4	4	NC	1	NC	1
312			min	-.004	2	-.008	3	0	1	0	1	NC	1	3501.069	4
313		5	max	.006	3	-.002	15	.033	4	1.273e-3	4	NC	1	NC	1
314			min	-.005	2	-.01	3	0	1	0	1	NC	1	2747.932	4
315		6	max	.007	3	-.002	15	.039	4	1.72e-3	4	NC	1	NC	1
316			min	-.007	2	-.012	3	0	1	0	1	8765.692	3	2293.991	4
317		7	max	.008	3	-.003	15	.045	4	2.167e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
318		min	-.008	2	-.014	3	0	1	0	1	7835.562	3	1987.516	4
319	8	max	.01	3	-.003	15	.051	4	2.615e-3	4	NC	1	NC	1
320		min	-.01	2	-.015	3	0	1	0	1	7286.175	3	1763.184	4
321	9	max	.011	3	-.003	15	.057	4	3.062e-3	4	NC	1	NC	1
322		min	-.011	2	-.016	3	0	1	0	1	6852.437	4	1588.148	4
323	10	max	.013	3	-.003	15	.062	4	3.509e-3	4	NC	1	NC	1
324		min	-.012	2	-.017	3	0	1	0	1	6640.865	4	1444.099	4
325	11	max	.014	3	-.003	15	.068	4	3.957e-3	4	NC	1	NC	1
326		min	-.014	2	-.017	3	0	1	0	1	6645.547	4	1320.133	4
327	12	max	.015	3	-.003	15	.074	4	4.404e-3	4	NC	1	NC	1
328		min	-.015	2	-.017	3	0	1	0	1	6871.784	4	1209.529	4
329	13	max	.017	3	-.003	15	.081	4	4.852e-3	4	NC	1	NC	1
330		min	-.016	2	-.016	3	0	1	0	1	7364.661	4	1108.139	4
331	14	max	.018	3	-.003	15	.089	4	5.299e-3	4	NC	1	NC	1
332		min	-.018	2	-.015	3	0	1	0	1	8231.93	4	1013.498	4
333	15	max	.02	3	-.002	15	.097	4	5.746e-3	4	NC	1	NC	1
334		min	-.019	2	-.014	3	0	1	0	1	9710.557	4	924.279	4
335	16	max	.021	3	-.002	15	.107	4	6.194e-3	4	NC	1	NC	1
336		min	-.021	2	-.013	3	0	1	0	1	NC	1	839.909	4
337	17	max	.022	3	0	2	.118	4	6.641e-3	4	NC	1	NC	1
338		min	-.022	2	-.011	3	0	1	0	1	NC	1	760.284	4
339	18	max	.024	3	0	2	.131	4	7.089e-3	4	NC	1	NC	1
340		min	-.023	2	-.009	3	0	1	0	1	NC	1	685.55	4
341	19	max	.025	3	.002	2	.146	4	7.536e-3	4	NC	1	NC	1
342		min	-.025	2	-.008	3	0	1	0	1	NC	1	615.935	4
343	M8	1	max	.004	2	.024	2	0	9.763e-4	4	NC	1	NC	1
344		min	0	15	-.026	3	-.146	4	0	1	NC	1	170.066	4
345	2	max	.004	2	.023	2	0	1	9.763e-4	4	NC	1	NC	1
346		min	0	15	-.025	3	-.135	4	0	1	NC	1	184.395	4
347	3	max	.004	2	.022	2	0	1	9.763e-4	4	NC	1	NC	1
348		min	0	15	-.023	3	-.123	4	0	1	NC	1	201.482	4
349	4	max	.004	2	.02	2	0	1	9.763e-4	4	NC	1	NC	1
350		min	0	15	-.022	3	-.112	4	0	1	NC	1	222.037	4
351	5	max	.003	2	.019	2	0	1	9.763e-4	4	NC	1	NC	1
352		min	0	15	-.02	3	-.1	4	0	1	NC	1	247.025	4
353	6	max	.003	2	.018	2	0	1	9.763e-4	4	NC	1	NC	1
354		min	0	15	-.019	3	-.089	4	0	1	NC	1	277.786	4
355	7	max	.003	2	.016	2	0	1	9.763e-4	4	NC	1	NC	1
356		min	0	15	-.018	3	-.078	4	0	1	NC	1	316.22	4
357	8	max	.003	2	.015	2	0	1	9.763e-4	4	NC	1	NC	1
358		min	0	15	-.016	3	-.068	4	0	1	NC	1	365.101	4
359	9	max	.002	2	.014	2	0	1	9.763e-4	4	NC	1	NC	1
360		min	0	15	-.015	3	-.058	4	0	1	NC	1	428.601	4
361	10	max	.002	2	.012	2	0	1	9.763e-4	4	NC	1	NC	1
362		min	0	15	-.013	3	-.048	4	0	1	NC	1	513.237	4
363	11	max	.002	2	.011	2	0	1	9.763e-4	4	NC	1	NC	1
364		min	0	15	-.012	3	-.039	4	0	1	NC	1	629.672	4
365	12	max	.002	2	.009	2	0	1	9.763e-4	4	NC	1	NC	1
366		min	0	15	-.01	3	-.031	4	0	1	NC	1	796.339	4
367	13	max	.001	2	.008	2	0	1	9.763e-4	4	NC	1	NC	1
368		min	0	15	-.009	3	-.024	4	0	1	NC	1	1047.429	4
369	14	max	.001	2	.007	2	0	1	9.763e-4	4	NC	1	NC	1
370		min	0	15	-.007	3	-.017	4	0	1	NC	1	1452.46	4
371	15	max	0	2	.005	2	0	1	9.763e-4	4	NC	1	NC	1
372		min	0	15	-.006	3	-.011	4	0	1	NC	1	2171.559	4
373	16	max	0	2	.004	2	0	1	9.763e-4	4	NC	1	NC	1
374		min	0	15	-.004	3	-.007	4	0	1	NC	1	3646.934	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
375	17	max	0	2	.003	2	0	1	9.763e-4	4	NC	1	NC	1
376		min	0	15	-.003	3	-.003	4	0	1	NC	1	7515.881	4
377	18	max	0	2	.001	2	0	1	9.763e-4	4	NC	1	NC	1
378		min	0	15	-.001	3	-.001	4	0	1	NC	1	NC	1
379	19	max	0	1	0	1	0	1	9.763e-4	4	NC	1	NC	1
380		min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.01	2	0	1.393e-3	4	NC	1	NC	1
382		min	-.01	3	-.016	3	-.442	4	1.243e-5	10	7425.377	2	174.136	4
383	2	max	.007	2	.009	2	0	10	1.426e-3	4	NC	1	NC	1
384		min	-.01	3	-.015	3	-.407	4	1.179e-5	10	8607.117	2	189.239	4
385	3	max	.006	2	.008	2	0	10	1.46e-3	4	NC	1	NC	1
386		min	-.009	3	-.015	3	-.372	4	1.114e-5	10	NC	1	207.113	4
387	4	max	.006	2	.006	2	0	10	1.493e-3	4	NC	1	NC	1
388		min	-.009	3	-.014	3	-.337	4	1.05e-5	10	NC	1	228.469	4
389	5	max	.006	2	.005	2	0	10	1.527e-3	4	NC	1	NC	1
390		min	-.008	3	-.014	3	-.303	4	9.851e-6	10	NC	1	254.273	4
391	6	max	.005	2	.004	2	0	12	1.56e-3	4	NC	1	NC	1
392		min	-.007	3	-.013	3	-.269	4	9.207e-6	10	NC	1	285.857	4
393	7	max	.005	2	.002	2	0	12	1.594e-3	4	NC	1	NC	1
394		min	-.007	3	-.013	3	-.237	4	8.562e-6	10	NC	1	325.1	4
395	8	max	.004	2	.001	2	0	12	1.627e-3	4	NC	1	NC	1
396		min	-.006	3	-.012	3	-.205	4	7.918e-6	10	NC	1	374.726	4
397	9	max	.004	2	0	2	0	12	1.66e-3	4	NC	1	NC	1
398		min	-.006	3	-.011	3	-.175	4	7.273e-6	10	NC	1	438.812	4
399	10	max	.004	2	0	2	0	12	1.694e-3	4	NC	1	NC	1
400		min	-.005	3	-.01	3	-.147	4	6.629e-6	10	NC	1	523.677	4
401	11	max	.003	2	0	2	0	12	1.727e-3	4	NC	1	NC	1
402		min	-.005	3	-.009	3	-.12	4	5.984e-6	10	NC	1	639.57	4
403	12	max	.003	2	-.001	2	0	12	1.761e-3	4	NC	1	NC	1
404		min	-.004	3	-.008	3	-.096	4	5.34e-6	10	NC	1	804.021	4
405	13	max	.002	2	-.002	15	0	12	1.794e-3	4	NC	1	NC	1
406		min	-.003	3	-.007	3	-.073	4	4.695e-6	10	NC	1	1049.134	4
407	14	max	.002	2	-.002	15	0	12	1.827e-3	4	NC	1	NC	1
408		min	-.003	3	-.006	3	-.054	4	4.051e-6	10	NC	1	1439.109	4
409	15	max	.002	2	-.001	15	0	12	1.861e-3	4	NC	1	NC	1
410		min	-.002	3	-.005	3	-.036	4	3.406e-6	10	NC	1	2118.602	4
411	16	max	.001	2	-.001	15	0	10	1.894e-3	4	NC	1	NC	1
412		min	-.002	3	-.004	3	-.022	4	2.762e-6	10	NC	1	3474.888	4
413	17	max	0	2	0	15	0	10	1.928e-3	4	NC	1	NC	1
414		min	-.001	3	-.003	4	-.011	4	2.117e-6	10	NC	1	6874.776	4
415	18	max	0	2	0	15	0	10	1.961e-3	4	NC	1	NC	1
416		min	0	3	-.002	4	-.004	4	1.472e-6	10	NC	1	NC	1
417	19	max	0	1	0	1	0	1	1.994e-3	4	NC	1	NC	1
418		min	0	1	0	1	0	1	8.279e-7	10	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-1.815e-7	10	NC	1	NC	1
420		min	0	1	0	1	0	1	-5.121e-4	4	NC	1	NC	1
421	2	max	0	3	0	15	.009	4	-1.243e-6	12	NC	1	NC	1
422		min	0	2	-.002	4	0	10	-6.057e-5	4	NC	1	9620.147	4
423	3	max	0	3	-.001	15	.018	4	3.925e-4	5	NC	1	NC	1
424		min	0	2	-.004	4	0	10	-2.286e-5	1	NC	1	5053.026	4
425	4	max	.001	3	-.002	15	.025	4	8.429e-4	5	NC	1	NC	1
426		min	-.001	2	-.006	4	0	10	-3.338e-5	1	NC	1	3532.989	4
427	5	max	.002	3	-.002	15	.032	4	1.294e-3	4	NC	1	NC	1
428		min	-.002	2	-.008	4	0	10	-4.389e-5	1	NC	1	2772.528	4
429	6	max	.002	3	-.003	15	.039	4	1.746e-3	4	NC	1	NC	1
430		min	-.002	2	-.01	4	0	10	-5.44e-5	1	9134.954	4	2313.669	4
431	7	max	.003	3	-.003	15	.045	4	2.197e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
432			min	-.002	2	-.012	4	0	1	-6.492e-5	1	7906.879	4	2003.368	4
433		8	max	.003	3	-.003	15	.051	4	2.649e-3	4	NC	2	NC	1
434			min	-.003	2	-.013	4	0	1	-7.543e-5	1	7151.435	4	1775.746	4
435		9	max	.004	3	-.003	15	.056	4	3.1e-3	4	NC	5	NC	1
436			min	-.003	2	-.014	4	0	1	-8.594e-5	1	6711.49	4	1597.695	4
437		10	max	.004	3	-.004	15	.062	4	3.552e-3	4	NC	5	NC	1
438			min	-.004	2	-.014	4	0	1	-9.646e-5	1	6511.476	4	1450.794	4
439		11	max	.005	3	-.004	15	.068	4	4.003e-3	4	NC	5	NC	1
440			min	-.004	2	-.014	4	0	1	-1.07e-4	1	6522.199	4	1324.108	4
441		12	max	.005	3	-.004	15	.074	4	4.455e-3	4	NC	5	NC	1
442			min	-.005	2	-.014	4	0	1	-1.175e-4	1	6749.626	4	1210.936	4
443		13	max	.006	3	-.003	15	.081	4	4.907e-3	4	NC	2	NC	1
444			min	-.005	2	-.013	4	-.001	1	-1.28e-4	1	7238.625	4	1107.18	4
445		14	max	.006	3	-.003	15	.089	4	5.358e-3	4	NC	1	NC	1
446			min	-.005	2	-.012	4	-.001	1	-1.385e-4	1	8095.617	4	1010.435	4
447		15	max	.006	3	-.003	15	.098	4	5.81e-3	4	NC	1	NC	1
448			min	-.006	2	-.01	4	-.002	1	-1.49e-4	1	9554.173	4	919.431	4
449		16	max	.007	3	-.002	15	.108	4	6.261e-3	4	NC	1	NC	1
450			min	-.006	2	-.008	4	-.002	1	-1.595e-4	1	NC	1	833.636	4
451		17	max	.007	3	-.002	15	.119	4	6.713e-3	4	NC	1	NC	1
452			min	-.007	2	-.006	4	-.003	1	-1.701e-4	1	NC	1	752.962	4
453		18	max	.008	3	-.001	10	.133	4	7.164e-3	4	NC	1	NC	1
454			min	-.007	2	-.004	3	-.003	1	-1.806e-4	1	NC	1	677.543	4
455		19	max	.008	3	0	2	.148	4	7.616e-3	4	NC	1	NC	1
456			min	-.007	2	-.003	3	-.004	1	-1.911e-4	1	NC	1	607.574	4
457	M12	1	max	.002	1	.007	2	.004	1	1.055e-3	5	NC	1	NC	2
458			min	0	12	-.009	3	-.148	4	-7.523e-5	1	NC	1	167.757	4
459		2	max	.002	1	.007	2	.004	1	1.055e-3	5	NC	1	NC	2
460			min	0	12	-.008	3	-.136	4	-7.523e-5	1	NC	1	181.88	4
461		3	max	.002	1	.006	2	.003	1	1.055e-3	5	NC	1	NC	2
462			min	0	12	-.008	3	-.125	4	-7.523e-5	1	NC	1	198.721	4
463		4	max	.001	1	.006	2	.003	1	1.055e-3	5	NC	1	NC	2
464			min	0	12	-.007	3	-.113	4	-7.523e-5	1	NC	1	218.98	4
465		5	max	.001	1	.006	2	.003	1	1.055e-3	5	NC	1	NC	2
466			min	0	12	-.007	3	-.102	4	-7.523e-5	1	NC	1	243.611	4
467		6	max	.001	1	.005	2	.002	1	1.055e-3	5	NC	1	NC	1
468			min	0	12	-.006	3	-.091	4	-7.523e-5	1	NC	1	273.932	4
469		7	max	.001	1	.005	2	.002	1	1.055e-3	5	NC	1	NC	1
470			min	0	12	-.006	3	-.08	4	-7.523e-5	1	NC	1	311.818	4
471		8	max	.001	1	.004	2	.002	1	1.055e-3	5	NC	1	NC	1
472			min	0	12	-.005	3	-.069	4	-7.523e-5	1	NC	1	360.001	4
473		9	max	0	1	.004	2	.002	1	1.055e-3	5	NC	1	NC	1
474			min	0	12	-.005	3	-.059	4	-7.523e-5	1	NC	1	422.593	4
475		10	max	0	1	.004	2	.001	1	1.055e-3	5	NC	1	NC	1
476			min	0	12	-.004	3	-.049	4	-7.523e-5	1	NC	1	506.021	4
477		11	max	0	1	.003	2	.001	1	1.055e-3	5	NC	1	NC	1
478			min	0	12	-.004	3	-.04	4	-7.523e-5	1	NC	1	620.793	4
479		12	max	0	1	.003	2	0	1	1.055e-3	5	NC	1	NC	1
480			min	0	12	-.003	3	-.032	4	-7.523e-5	1	NC	1	785.077	4
481		13	max	0	1	.002	2	0	1	1.055e-3	5	NC	1	NC	1
482			min	0	12	-.003	3	-.024	4	-7.523e-5	1	NC	1	1032.576	4
483		14	max	0	1	.002	2	0	1	1.055e-3	5	NC	1	NC	1
484			min	0	12	-.002	3	-.017	4	-7.523e-5	1	NC	1	1431.809	4
485		15	max	0	1	.002	2	0	1	1.055e-3	5	NC	1	NC	1
486			min	0	12	-.002	3	-.012	4	-7.523e-5	1	NC	1	2140.6	4
487		16	max	0	1	.001	2	0	1	1.055e-3	5	NC	1	NC	1
488			min	0	12	-.001	3	-.007	4	-7.523e-5	1	NC	1	3594.794	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
489	17	max	0	1	0	2	0	1	1.055e-3	5	NC	1	NC	1
490		min	0	12	0	3	-.003	4	-7.523e-5	1	NC	1	7408.078	4
491	18	max	0	1	0	2	0	1	1.055e-3	5	NC	1	NC	1
492		min	0	12	0	3	-.001	4	-7.523e-5	1	NC	1	NC	1
493	19	max	0	1	0	1	0	1	1.055e-3	5	NC	1	NC	1
494		min	0	1	0	1	0	1	-7.523e-5	1	NC	1	NC	1
495	M1	1	max	.01	.112	.466	.466	4	5.887e-3	2	NC	1	NC	1
496		min	-.006	2	-.03	3	0	12	-1.423e-2	3	NC	1	NC	1
497	2	max	.01	3	.052	.453	.453	4	4.432e-3	4	NC	4	NC	1
498		min	-.006	2	-.01	3	-.003	1	-7.043e-3	3	1913.373	2	NC	1
499	3	max	.01	3	.017	.439	.439	4	7.961e-3	4	NC	5	NC	1
500		min	-.006	2	-.012	2	-.004	1	-9.621e-5	3	927.688	2	8327.915	5
501	4	max	.01	3	.058	.424	.424	4	6.812e-3	4	NC	5	NC	1
502		min	-.006	2	-.083	2	-.004	1	-3.31e-3	3	590.724	2	6077.478	5
503	5	max	.01	3	.108	.409	.409	4	5.798e-3	2	NC	5	NC	1
504		min	-.006	2	-.156	2	-.003	1	-6.524e-3	3	429.522	2	4949.972	5
505	6	max	.01	3	.16	.393	.393	4	8.687e-3	2	NC	5	NC	1
506		min	-.006	2	-.227	2	-.001	1	-9.738e-3	3	340.257	2	4258.905	5
507	7	max	.009	3	.21	.377	.377	4	1.158e-2	2	NC	15	NC	1
508		min	-.006	2	-.289	2	0	3	-1.295e-2	3	287.332	2	3751.12	4
509	8	max	.009	3	.252	.361	.361	4	1.446e-2	2	NC	15	NC	1
510		min	-.006	2	-.339	2	0	12	-1.617e-2	3	255.923	2	3352.325	4
511	9	max	.009	3	.278	.345	.345	4	1.639e-2	2	NC	15	NC	1
512		min	-.006	2	-.37	2	0	1	-1.659e-2	3	239.532	2	3066.011	4
513	10	max	.009	3	.288	.326	.326	4	1.769e-2	2	NC	15	NC	1
514		min	-.006	2	-.38	2	0	10	-1.515e-2	3	234.758	2	2953.081	4
515	11	max	.009	3	.281	.306	.306	4	1.899e-2	2	NC	15	NC	1
516		min	-.006	2	-.369	2	0	12	-1.371e-2	3	240.498	2	2965.242	4
517	12	max	.008	3	.257	.285	.285	4	1.832e-2	2	NC	15	NC	1
518		min	-.005	2	-.337	2	0	1	-1.19e-2	3	258.81	2	3098.987	4
519	13	max	.008	3	.219	.26	.26	4	1.469e-2	2	NC	15	NC	1
520		min	-.005	2	-.284	2	0	1	-9.525e-3	3	294.25	2	3558.543	4
521	14	max	.008	3	.171	.233	.233	4	1.106e-2	2	NC	5	NC	1
522		min	-.005	2	-.219	2	0	12	-7.149e-3	3	354.847	2	4573.626	4
523	15	max	.008	3	.117	.205	.205	4	7.429e-3	2	NC	5	NC	1
524		min	-.005	2	-.147	2	0	12	-4.773e-3	3	459.164	2	6801.281	4
525	16	max	.007	3	.06	.177	.177	4	6.411e-3	4	NC	5	NC	1
526		min	-.005	2	-.074	2	0	12	-2.397e-3	3	652.332	2	NC	1
527	17	max	.007	3	.006	.151	.151	4	7.47e-3	4	NC	5	NC	1
528		min	-.005	2	-.007	2	0	12	-2.111e-5	3	1065.234	2	NC	1
529	18	max	.007	3	.049	.128	.128	4	5.239e-3	2	NC	4	NC	1
530		min	-.005	2	-.043	3	0	12	-2.153e-3	3	2260.14	2	NC	1
531	19	max	.007	3	.099	.109	.109	4	1.051e-2	2	NC	1	NC	1
532		min	-.005	2	-.089	3	0	1	-4.391e-3	3	NC	1	NC	1
533	M5	1	max	.031	.204	.466	.466	4	0	1	NC	1	NC	1
534		min	-.022	2	-.006	3	0	1	-9.334e-6	4	NC	1	NC	1
535	2	max	.031	3	.09	.456	.456	4	4.085e-3	4	NC	5	NC	1
536		min	-.022	2	.002	15	0	1	0	1	1024.298	2	NC	1
537	3	max	.031	3	.052	.443	.443	4	8.053e-3	4	NC	5	NC	1
538		min	-.022	2	-.038	2	0	1	0	1	481.323	2	6864.398	4
539	4	max	.03	3	.138	.428	.428	4	6.56e-3	4	NC	5	NC	1
540		min	-.022	2	-.191	2	0	1	0	1	294.136	2	5349.417	4
541	5	max	.029	3	.256	.411	.411	4	5.067e-3	4	NC	15	NC	1
542		min	-.021	2	-.356	2	0	1	0	1	206.765	2	4631.958	4
543	6	max	.029	3	.388	.394	.394	4	3.574e-3	4	8563.133	15	NC	1
544		min	-.021	2	-.521	2	0	1	0	1	159.674	2	4185.913	4
545	7	max	.028	3	.517	.377	.377	4	2.082e-3	4	7071.393	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
546		min	-.02	2	-.671	2	0	1	0	1	132.372	2	3810.882	4
547	8	max	.027	3	.624	3	.361	4	5.89e-4	4	6206.205	15	NC	1
548		min	-.02	2	-.791	2	0	1	0	1	116.453	2	3412.878	4
549	9	max	.027	3	.693	3	.345	4	0	1	5763.357	15	NC	1
550		min	-.02	2	-.867	2	0	1	-7.029e-6	5	108.273	2	3056.573	4
551	10	max	.026	3	.717	3	.326	4	0	1	5630.135	15	NC	1
552		min	-.019	2	-.893	2	0	1	-6.836e-6	5	105.896	2	2973.797	4
553	11	max	.026	3	.697	3	.306	4	0	1	5763.872	15	NC	1
554		min	-.019	2	-.867	2	0	1	-6.642e-6	5	108.753	2	3002.91	4
555	12	max	.025	3	.636	3	.285	4	5.253e-4	4	6207.385	15	NC	1
556		min	-.019	2	-.786	2	0	1	0	1	118.051	2	3043.746	4
557	13	max	.024	3	.539	3	.261	4	1.857e-3	4	7073.688	15	NC	1
558		min	-.019	2	-.658	2	0	1	0	1	136.571	2	3491.84	4
559	14	max	.024	3	.416	3	.232	4	3.188e-3	4	8567.45	15	NC	1
560		min	-.018	2	-.499	2	0	1	0	1	169.258	2	4724.193	4
561	15	max	.023	3	.281	3	.203	4	4.519e-3	4	NC	15	NC	1
562		min	-.018	2	-.328	2	0	1	0	1	227.965	2	8135.233	4
563	16	max	.022	3	.144	3	.173	4	5.851e-3	4	NC	5	NC	1
564		min	-.018	2	-.162	2	0	1	0	1	342.991	2	NC	1
565	17	max	.022	3	.017	3	.146	4	7.182e-3	4	NC	5	NC	1
566		min	-.017	2	-.02	2	0	1	0	1	604.912	2	NC	1
567	18	max	.022	3	.082	2	.125	4	3.645e-3	4	NC	5	NC	1
568		min	-.017	2	-.089	3	0	1	0	1	1346.341	3	NC	1
569	19	max	.022	3	.164	2	.109	4	0	1	NC	1	NC	1
570		min	-.017	2	-.183	3	0	1	-5.775e-6	4	NC	1	NC	1
571	M9	1	max	.01	.112	2	.466	4	1.423e-2	3	NC	1	NC	1
572		min	-.006	2	-.03	3	0	1	-5.887e-3	2	NC	1	NC	1
573	2	max	.01	3	.052	2	.455	4	7.043e-3	3	NC	4	NC	1
574		min	-.006	2	-.01	3	0	10	-2.887e-3	2	1913.373	2	NC	1
575	3	max	.01	3	.017	3	.442	4	8.029e-3	4	NC	5	NC	1
576		min	-.006	2	-.012	2	0	10	-2.39e-5	10	927.688	2	7244.957	4
577	4	max	.01	3	.058	3	.427	4	6.379e-3	5	NC	5	NC	1
578		min	-.006	2	-.083	2	0	10	-2.909e-3	2	590.724	2	5513.612	4
579	5	max	.01	3	.108	3	.411	4	6.524e-3	3	NC	5	NC	1
580		min	-.006	2	-.156	2	0	10	-5.798e-3	2	429.522	2	4670.101	4
581	6	max	.01	3	.16	3	.394	4	9.738e-3	3	NC	5	NC	1
582		min	-.006	2	-.227	2	0	10	-8.687e-3	2	340.257	2	4150.173	4
583	7	max	.009	3	.21	3	.377	4	1.295e-2	3	NC	15	NC	1
584		min	-.006	2	-.289	2	0	1	-1.158e-2	2	287.332	2	3750.912	4
585	8	max	.009	3	.252	3	.361	4	1.617e-2	3	NC	15	NC	1
586		min	-.006	2	-.339	2	0	1	-1.446e-2	2	255.923	2	3378.854	4
587	9	max	.009	3	.278	3	.345	4	1.659e-2	3	NC	15	NC	1
588		min	-.006	2	-.37	2	0	12	-1.639e-2	2	239.532	2	3058.021	4
589	10	max	.009	3	.288	3	.326	4	1.515e-2	3	NC	15	NC	1
590		min	-.006	2	-.38	2	0	1	-1.769e-2	2	234.758	2	2954.206	4
591	11	max	.009	3	.281	3	.306	4	1.371e-2	3	NC	15	NC	1
592		min	-.006	2	-.369	2	0	1	-1.899e-2	2	240.498	2	2974.507	4
593	12	max	.008	3	.257	3	.285	4	1.19e-2	3	NC	15	NC	1
594		min	-.005	2	-.337	2	0	12	-1.832e-2	2	258.81	2	3076.799	4
595	13	max	.008	3	.219	3	.26	4	9.525e-3	3	NC	15	NC	1
596		min	-.005	2	-.284	2	0	10	-1.469e-2	2	294.25	2	3555.202	4
597	14	max	.008	3	.171	3	.232	4	7.149e-3	3	NC	5	NC	1
598		min	-.005	2	-.219	2	-.001	1	-1.106e-2	2	354.847	2	4693.369	5
599	15	max	.008	3	.117	3	.203	4	4.773e-3	3	NC	5	NC	1
600		min	-.005	2	-.147	2	-.002	1	-7.429e-3	2	459.164	2	7348.808	5
601	16	max	.007	3	.06	3	.175	4	5.862e-3	5	NC	5	NC	1
602		min	-.005	2	-.074	2	-.004	1	-3.8e-3	2	652.332	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
603	17	max	.007	3	.006	3	.148	4	7.301e-3	4	NC	5	NC	1
604		min	-.005	2	-.007	2	-.004	1	-2.816e-4	1	1065.234	2	NC	1
605	18	max	.007	3	.049	2	.126	4	3.607e-3	5	NC	4	NC	1
606		min	-.005	2	-.043	3	-.003	1	-5.239e-3	2	2260.14	2	NC	1
607	19	max	.007	3	.099	2	.109	4	4.391e-3	3	NC	1	NC	1
608		min	-.005	2	-.089	3	0	12	-1.051e-2	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298



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Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

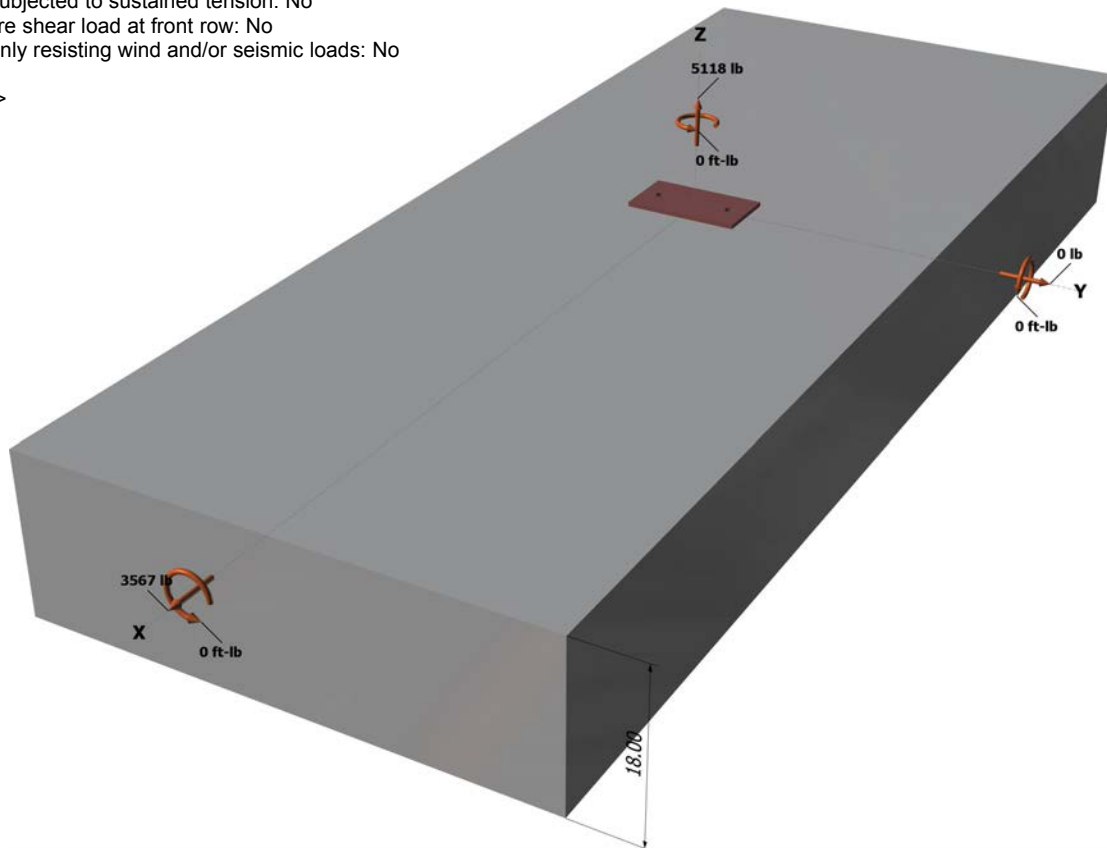
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

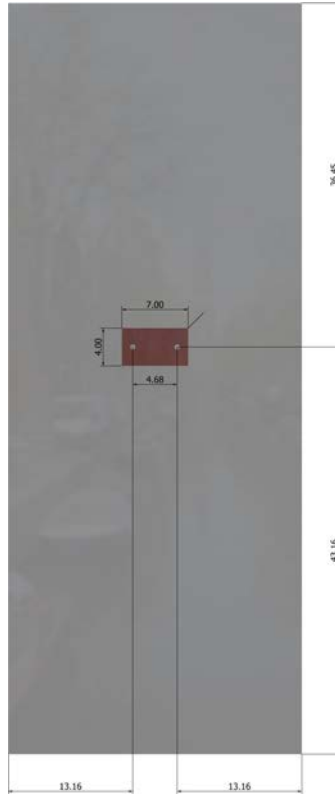
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Engineer:	HCV	Page:	2/5
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Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

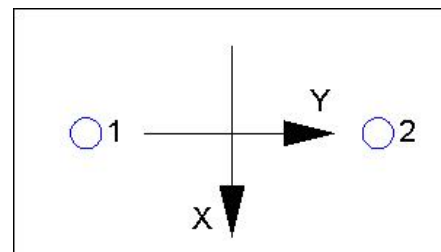
Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5118  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
<b>Adhesive</b>	<b>5118</b>	<b>8093</b>	<b>0.63</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1784</b>	<b>3156</b>	<b>0.57</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.