



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-10	35° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height ≤	15 ft	Importance Category = II
Peak Velocity Pressure, q_z =	26.53 psf	Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25	ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .
S_{DS} =	0.00	C_s = 0	
S_1 =	0.00	ρ = 1.3	
S_{D1} =	0.00	Ω = 1.25	
T_a =	0.00	C_d = 1.25	

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

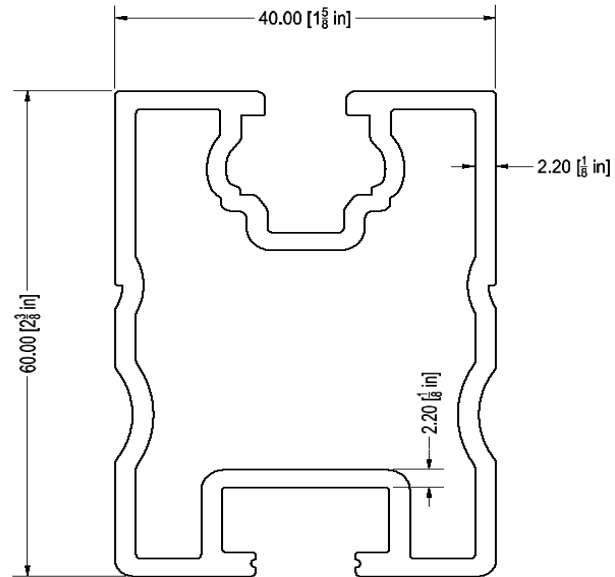
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	63 in
ΦF_{ty} STRONG-AXIS =	29.20 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	0.570 k-ft
M_z =	0.053 k-ft
$M_{y \text{ allowable}}$ =	1.243 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	52%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.59 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.613 k-ft
M_z =	0.000 k-ft
P_n =	0.282 k
$M_{y \text{ allowable}}$ =	1.452 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	44%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.790 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

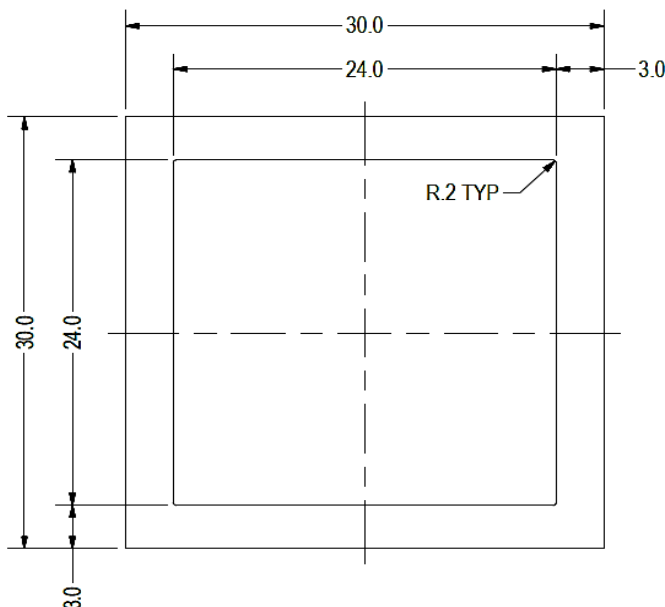
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.778 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	20%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.678 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	15%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.003 k-ft
P_n =	0.093 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	7%



A cross brace kit is required every 27 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

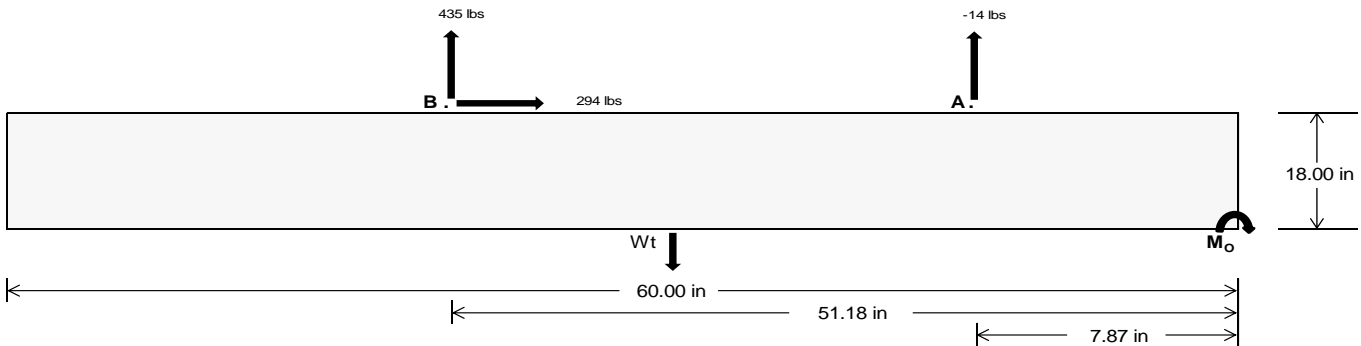
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	25.26	1886.98	k
Compressive Load =	1027.54	1292.84	k
Lateral Load =	2.31	1273.65	k
Moment (Weak Axis) =	0.00	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 27418.9$ in-lbs
Resisting Force Required = 913.96 lbs
S.F. = 1.67
Weight Required = 1523.27 lbs
Minimum Width = 21 in
Weight Provided = 1903.13 lbs

Sliding

Force = 293.75 lbs
Friction = 0.4
Weight Required = 734.38 lbs
Resisting Weight = 1903.13 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 293.75 lbs
Cohesion = 130 psf
Area = 8.75 ft²
Resisting = 951.56 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width	21 in	22 in	23 in	24 in
P_{ftg}	1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
F_A	378 lbs	378 lbs	378 lbs	378 lbs	337 lbs	337 lbs	337 lbs	337 lbs	498 lbs	498 lbs	498 lbs	498 lbs	28 lbs	28 lbs	28 lbs	28 lbs
F_B	250 lbs	250 lbs	250 lbs	250 lbs	552 lbs	552 lbs	552 lbs	552 lbs	575 lbs	575 lbs	575 lbs	575 lbs	-869 lbs	-869 lbs	-869 lbs	-869 lbs
F_V	44 lbs	44 lbs	44 lbs	44 lbs	533 lbs	533 lbs	533 lbs	533 lbs	429 lbs	429 lbs	429 lbs	429 lbs	-588 lbs	-588 lbs	-588 lbs	-588 lbs
P_{total}	2531 lbs	2621 lbs	2712 lbs	2802 lbs	2792 lbs	2883 lbs	2973 lbs	3064 lbs	2976 lbs	3066 lbs	3157 lbs	3248 lbs	301 lbs	355 lbs	410 lbs	464 lbs
M	322 lbs-ft	322 lbs-ft	322 lbs-ft	322 lbs-ft	447 lbs-ft	447 lbs-ft	447 lbs-ft	447 lbs-ft	547 lbs-ft	547 lbs-ft	547 lbs-ft	547 lbs-ft	705 lbs-ft	705 lbs-ft	705 lbs-ft	705 lbs-ft
e	0.13 ft	0.12 ft	0.12 ft	0.12 ft	0.16 ft	0.16 ft	0.15 ft	0.15 ft	0.18 ft	0.18 ft	0.17 ft	0.17 ft	2.34 ft	1.98 ft	1.72 ft	1.52 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	245.0 psf	243.8 psf	242.6 psf	241.6 psf	257.7 psf	255.9 psf	254.2 psf	252.7 psf	265.1 psf	263.0 psf	261.0 psf	259.2 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	333.4 psf	328.1 psf	323.3 psf	318.9 psf	380.4 psf	373.0 psf	366.3 psf	360.1 psf	415.1 psf	406.1 psf	397.9 psf	390.4 psf	727.8 psf	250.3 psf	182.8 psf	157.7 psf

Maximum Bearing Pressure = 728 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

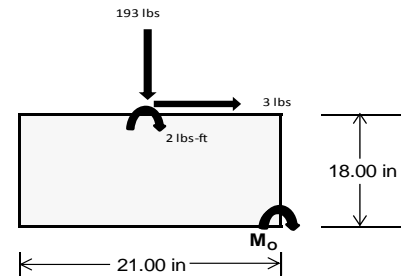
Overturning Check

$M_o = 162.6$ ft-lbs
 Resisting Force Required = 185.88 lbs
 S.F. = 1.67
 Weight Required = 309.80 lbs
 Minimum Width = 21 in
 Weight Provided = 1903.13 lbs

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	66 lbs	160 lbs	62 lbs	193 lbs	526 lbs	189 lbs	19 lbs	47 lbs	18 lbs
F_v	0 lbs	0 lbs	0 lbs	3 lbs	2 lbs	1 lbs	0 lbs	0 lbs	0 lbs
P_{total}	2422 lbs	2516 lbs	2418 lbs	2436 lbs	2769 lbs	2432 lbs	708 lbs	736 lbs	707 lbs
M	1 lbs-ft	0 lbs-ft	0 lbs-ft	6 lbs-ft	4 lbs-ft	1 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.29 ft	1.75 ft	1.75 ft	1.74 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft	1.75 ft
f_{min}	276.5 sqft	287.4 sqft	276.3 sqft	275.9 sqft	315.1 sqft	277.7 sqft	80.9 sqft	84.0 sqft	80.8 sqft
f_{max}	277.1 psf	287.8 psf	276.5 psf	280.8 psf	317.8 psf	278.3 psf	81.0 psf	84.1 psf	80.8 psf



Maximum Bearing Pressure = 318 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.597 k
Allowable Uplift =	1.214 k
Utilization =	<u>49%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.118 k
Allowable Uplift =	1.116 k
Utilization =	<u>100%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.790 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>

Diagonal Strut

Maximum Axial Load =	0.778 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>14%</u>



Rear Strut

Maximum Axial Load =	1.164 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

Bracing

Maximum Axial Load =	0.093 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>1%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.018 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$164.048$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 63.00 \text{ in}$$

$$J = 0.255$$

$$170.354$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.1$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.2 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.243 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.20 \\
 &22.3976 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.20 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.6 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.6 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.452 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 1.81475$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83406$$

$$\phi_{FL} = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi_{FL} = 8.86409 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 8.86 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 4.45 \text{ kips}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	262.137	2	301.972	2	-.001	15	0	15	0	1	0	1
2		min	-312.232	3	-448.159	3	-.122	3	0	3	0	1	0	1
3	N7	max	.025	3	306.059	1	-.041	15	0	15	0	1	0	1
4		min	-.149	2	12.559	15	-.848	1	-.001	1	0	1	0	1
5	N15	max	.18	3	790.416	1	.415	1	0	1	0	1	0	1
6		min	-1.475	2	27.641	15	-.642	3	-.001	3	0	1	0	1
7	N16	max	903.556	2	994.492	2	0	10	0	1	0	1	0	1
8		min	-979.73	3	-1451.526	3	-75.632	3	0	3	0	1	0	1
9	N23	max	.026	3	305.936	1	1.775	1	.003	1	0	1	0	1
10		min	-.149	2	12.681	15	.09	15	0	15	0	1	0	1
11	N24	max	262.171	2	304.866	2	76.243	3	0	1	0	1	0	1
12		min	-312.623	3	-446.7	3	.005	10	0	3	0	1	0	1
13	Totals:	max	1426.092	2	2738.678	2	0	3						
14		min	-1604.354	3	-2155.723	3	0	1						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	212.663	1	.678	4	.24	1	0	15	0	15	0	1
2			min	-370.314	3	.159	15	-.048	3	0	1	0	1	0	1
3		2	max	212.798	1	.621	4	.24	1	0	15	0	15	0	15
4			min	-370.213	3	.146	15	-.048	3	0	1	0	1	0	4
5		3	max	212.933	1	.563	4	.24	1	0	15	0	15	0	15
6			min	-370.111	3	.132	15	-.048	3	0	1	0	1	0	4
7		4	max	213.068	1	.506	4	.24	1	0	15	0	15	0	15
8			min	-370.01	3	.119	15	-.048	3	0	1	0	3	0	4
9		5	max	213.203	1	.448	4	.24	1	0	15	0	9	0	15
10			min	-369.909	3	.105	15	-.048	3	0	1	0	3	0	4
11		6	max	213.337	1	.391	4	.24	1	0	15	0	1	0	15
12			min	-369.808	3	.092	15	-.048	3	0	1	0	3	0	4
13		7	max	213.472	1	.333	4	.24	1	0	15	0	1	0	15
14			min	-369.707	3	.078	15	-.048	3	0	1	0	3	0	4
15		8	max	213.607	1	.276	4	.24	1	0	15	0	1	0	15
16			min	-369.606	3	.065	15	-.048	3	0	1	0	3	0	4
17		9	max	213.742	1	.218	4	.24	1	0	15	0	1	0	15
18			min	-369.505	3	.051	15	-.048	3	0	1	0	3	0	4
19		10	max	213.877	1	.161	4	.24	1	0	15	0	1	0	15
20			min	-369.403	3	.038	15	-.048	3	0	1	0	3	0	4
21		11	max	214.012	1	.11	2	.24	1	0	15	0	1	0	15
22			min	-369.302	3	.016	12	-.048	3	0	1	0	3	0	4
23		12	max	214.147	1	.065	2	.24	1	0	15	0	1	0	15
24			min	-369.201	3	-.014	3	-.048	3	0	1	0	3	0	4
25		13	max	214.281	1	.02	2	.24	1	0	15	0	1	0	15
26			min	-369.1	3	-.047	3	-.048	3	0	1	0	3	0	4
27		14	max	214.416	1	-.016	15	.24	1	0	15	0	1	0	15
28			min	-368.999	3	-.081	3	-.048	3	0	1	0	3	0	4
29		15	max	214.551	1	-.03	15	.24	1	0	15	0	1	0	15
30			min	-368.898	3	-.127	4	-.048	3	0	1	0	3	0	4
31		16	max	214.686	1	-.043	15	.24	1	0	15	0	1	0	15
32			min	-368.796	3	-.184	4	-.048	3	0	1	0	3	0	4
33		17	max	214.821	1	-.057	15	.24	1	0	15	0	1	0	15
34			min	-368.695	3	-.242	4	-.048	3	0	1	0	3	0	4
35		18	max	214.956	1	-.07	15	.24	1	0	15	0	1	0	15
36			min	-368.594	3	-.299	4	-.048	3	0	1	0	3	0	4
37		19	max	215.091	1	-.084	15	.24	1	0	15	0	1	0	15



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38			min	-368.493	3	-.357	4	-.048	3	0	1	0	3	0	4
39	M3	1	max	220.48	2	1.735	4	-.013	15	0	15	0	1	0	4
40			min	-219.164	3	.408	15	-.27	1	0	1	0	15	0	15
41		2	max	220.41	2	1.559	4	-.013	15	0	15	0	1	0	2
42			min	-219.217	3	.367	15	-.27	1	0	1	0	15	0	3
43		3	max	220.34	2	1.383	4	-.013	15	0	15	0	1	0	2
44			min	-219.269	3	.325	15	-.27	1	0	1	0	15	0	3
45		4	max	220.27	2	1.206	4	-.013	15	0	15	0	1	0	15
46			min	-219.322	3	.284	15	-.27	1	0	1	0	15	0	4
47		5	max	220.2	2	1.03	4	-.013	15	0	15	0	1	0	15
48			min	-219.374	3	.242	15	-.27	1	0	1	0	15	0	4
49		6	max	220.13	2	.854	4	-.013	15	0	15	0	1	0	15
50			min	-219.427	3	.201	15	-.27	1	0	1	0	15	0	4
51		7	max	220.06	2	.677	4	-.013	15	0	15	0	1	0	15
52			min	-219.479	3	.159	15	-.27	1	0	1	0	15	0	4
53		8	max	219.99	2	.501	4	-.013	15	0	15	0	1	0	15
54			min	-219.532	3	.118	15	-.27	1	0	1	0	15	-.001	4
55		9	max	219.92	2	.324	4	-.013	15	0	15	0	1	0	15
56			min	-219.584	3	.076	15	-.27	1	0	1	0	15	-.001	4
57		10	max	219.85	2	.148	4	-.013	15	0	15	0	1	0	15
58			min	-219.637	3	.035	15	-.27	1	0	1	0	15	-.001	4
59		11	max	219.78	2	.005	2	-.013	15	0	15	0	1	0	15
60			min	-219.689	3	-.054	3	-.27	1	0	1	0	15	-.001	4
61		12	max	219.71	2	-.048	15	-.013	15	0	15	0	1	0	15
62			min	-219.742	3	-.205	4	-.27	1	0	1	0	15	-.001	4
63		13	max	219.64	2	-.089	15	-.013	15	0	15	0	1	0	15
64			min	-219.794	3	-.381	4	-.27	1	0	1	0	15	-.001	4
65		14	max	219.57	2	-.131	15	-.013	15	0	15	0	1	0	15
66			min	-219.847	3	-.557	4	-.27	1	0	1	0	15	-.001	4
67		15	max	219.5	2	-.172	15	-.013	15	0	15	0	1	0	15
68			min	-219.899	3	-.734	4	-.27	1	0	1	0	15	0	4
69		16	max	219.43	2	-.214	15	-.013	15	0	15	0	1	0	15
70			min	-219.952	3	-.91	4	-.27	1	0	1	0	10	0	4
71		17	max	219.36	2	-.255	15	-.013	15	0	15	0	15	0	15
72			min	-220.004	3	-1.087	4	-.27	1	0	1	0	2	0	4
73		18	max	219.29	2	-.297	15	-.013	15	0	15	0	15	0	15
74			min	-220.057	3	-1.263	4	-.27	1	0	1	0	1	0	4
75		19	max	219.22	2	-.338	15	-.013	15	0	15	0	15	0	1
76			min	-220.109	3	-1.439	4	-.27	1	0	1	0	1	0	1
77	M4	1	max	304.895	1	0	1	-.041	15	0	1	0	3	0	1
78			min	12.208	15	0	1	-.893	1	0	1	0	2	0	1
79		2	max	304.959	1	0	1	-.041	15	0	1	0	15	0	1
80			min	12.227	15	0	1	-.893	1	0	1	0	1	0	1
81		3	max	305.024	1	0	1	-.041	15	0	1	0	15	0	1
82			min	12.247	15	0	1	-.893	1	0	1	0	1	0	1
83		4	max	305.089	1	0	1	-.041	15	0	1	0	15	0	1
84			min	12.267	15	0	1	-.893	1	0	1	0	1	0	1
85		5	max	305.153	1	0	1	-.041	15	0	1	0	15	0	1
86			min	12.286	15	0	1	-.893	1	0	1	0	1	0	1
87		6	max	305.218	1	0	1	-.041	15	0	1	0	15	0	1
88			min	12.306	15	0	1	-.893	1	0	1	0	1	0	1
89		7	max	305.283	1	0	1	-.041	15	0	1	0	15	0	1
90			min	12.325	15	0	1	-.893	1	0	1	0	1	0	1
91		8	max	305.348	1	0	1	-.041	15	0	1	0	15	0	1
92			min	12.345	15	0	1	-.893	1	0	1	0	1	0	1
93		9	max	305.412	1	0	1	-.041	15	0	1	0	15	0	1
94			min	12.364	15	0	1	-.893	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	305.477	1	0	1	-.041	15	0	1	0	15	0	1
96		min	12.384	15	0	1	-.893	1	0	1	0	1	0	1
97	11	max	305.542	1	0	1	-.041	15	0	1	0	15	0	1
98		min	12.403	15	0	1	-.893	1	0	1	0	1	0	1
99	12	max	305.606	1	0	1	-.041	15	0	1	0	15	0	1
100		min	12.423	15	0	1	-.893	1	0	1	0	1	0	1
101	13	max	305.671	1	0	1	-.041	15	0	1	0	15	0	1
102		min	12.442	15	0	1	-.893	1	0	1	0	1	0	1
103	14	max	305.736	1	0	1	-.041	15	0	1	0	15	0	1
104		min	12.462	15	0	1	-.893	1	0	1	-.001	1	0	1
105	15	max	305.801	1	0	1	-.041	15	0	1	0	15	0	1
106		min	12.481	15	0	1	-.893	1	0	1	-.001	1	0	1
107	16	max	305.865	1	0	1	-.041	15	0	1	0	15	0	1
108		min	12.501	15	0	1	-.893	1	0	1	-.001	1	0	1
109	17	max	305.93	1	0	1	-.041	15	0	1	0	15	0	1
110		min	12.52	15	0	1	-.893	1	0	1	-.001	1	0	1
111	18	max	305.995	1	0	1	-.041	15	0	1	0	15	0	1
112		min	12.54	15	0	1	-.893	1	0	1	-.001	1	0	1
113	19	max	306.059	1	0	1	-.041	15	0	1	0	15	0	1
114		min	12.559	15	0	1	-.893	1	0	1	-.001	1	0	1
115	M6	1	max	675.227	1	.681	.06	9	0	3	0	3	0	1
116		min	-1163.949	3	.16	15	-.215	3	0	10	0	9	0	1
117	2	max	675.362	1	.623	4	.06	9	0	3	0	3	0	15
118		min	-1163.847	3	.146	15	-.215	3	0	10	0	9	0	4
119	3	max	675.497	1	.566	4	.06	9	0	3	0	3	0	15
120		min	-1163.746	3	.133	15	-.215	3	0	10	0	9	0	4
121	4	max	675.631	1	.508	4	.06	9	0	3	0	3	0	15
122		min	-1163.645	3	.119	15	-.215	3	0	10	0	10	0	4
123	5	max	675.766	1	.451	4	.06	9	0	3	0	3	0	15
124		min	-1163.544	3	.099	12	-.215	3	0	10	0	2	0	4
125	6	max	675.901	1	.402	2	.06	9	0	3	0	1	0	15
126		min	-1163.443	3	.076	12	-.215	3	0	10	0	2	0	4
127	7	max	676.036	1	.357	2	.06	9	0	3	0	1	0	15
128		min	-1163.342	3	.054	12	-.215	3	0	10	0	2	0	4
129	8	max	676.171	1	.312	2	.06	9	0	3	0	1	0	12
130		min	-1163.241	3	.032	12	-.215	3	0	10	0	3	0	4
131	9	max	676.306	1	.267	2	.06	9	0	3	0	1	0	12
132		min	-1163.139	3	.001	3	-.215	3	0	10	0	3	0	4
133	10	max	676.441	1	.223	2	.06	9	0	3	0	1	0	12
134		min	-1163.038	3	-.032	3	-.215	3	0	10	0	3	0	2
135	11	max	676.576	1	.178	2	.06	9	0	3	0	1	0	12
136		min	-1162.937	3	-.066	3	-.215	3	0	10	0	3	0	2
137	12	max	676.71	1	.133	2	.06	9	0	3	0	1	0	12
138		min	-1162.836	3	-.099	3	-.215	3	0	10	0	3	0	2
139	13	max	676.845	1	.088	2	.06	9	0	3	0	1	0	12
140		min	-1162.735	3	-.133	3	-.215	3	0	10	0	3	0	2
141	14	max	676.98	1	.043	2	.06	9	0	3	0	1	0	12
142		min	-1162.634	3	-.167	3	-.215	3	0	10	0	3	0	2
143	15	max	677.115	1	-.001	2	.06	9	0	3	0	1	0	12
144		min	-1162.532	3	-.2	3	-.215	3	0	10	0	3	0	2
145	16	max	677.25	1	-.043	15	.06	9	0	3	0	1	0	3
146		min	-1162.431	3	-.234	3	-.215	3	0	10	0	3	0	2
147	17	max	677.385	1	-.056	15	.06	9	0	3	0	1	0	3
148		min	-1162.33	3	-.267	3	-.215	3	0	10	0	3	0	2
149	18	max	677.52	1	-.07	15	.06	9	0	3	0	1	0	3
150		min	-1162.229	3	-.301	3	-.215	3	0	10	0	3	0	2
151	19	max	677.654	1	-.083	15	.06	9	0	3	0	1	0	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1162.128	3	-.354	4	-.215	3	0	10	0	3	0	2
153	M7	1	max	778.215	2	1.739	4	.041	3	0	1	0	2	2
154		min	-668.462	3	.409	15	-.017	2	0	3	0	3	0	3
155		2	max	778.145	2	1.563	4	.041	3	0	1	0	2	2
156		min	-668.514	3	.367	15	-.017	2	0	3	0	3	0	3
157		3	max	778.075	2	1.387	4	.041	3	0	1	0	2	2
158		min	-668.567	3	.326	15	-.017	2	0	3	0	3	0	3
159		4	max	778.005	2	1.21	4	.041	3	0	1	0	2	2
160		min	-668.619	3	.284	15	-.017	2	0	3	0	3	0	3
161		5	max	777.935	2	1.034	4	.041	3	0	1	0	1	15
162		min	-668.672	3	.243	15	-.017	2	0	3	0	3	0	3
163		6	max	777.865	2	.857	4	.041	3	0	1	0	1	15
164		min	-668.724	3	.201	15	-.017	2	0	3	0	3	0	4
165		7	max	777.795	2	.681	4	.041	3	0	1	0	1	15
166		min	-668.777	3	.16	15	-.017	2	0	3	0	3	0	4
167		8	max	777.725	2	.505	4	.041	3	0	1	0	1	15
168		min	-668.829	3	.118	15	-.017	2	0	3	0	3	-.001	4
169		9	max	777.655	2	.346	2	.041	3	0	1	0	1	15
170		min	-668.882	3	.057	12	-.017	2	0	3	0	3	-.001	4
171		10	max	777.585	2	.208	2	.041	3	0	1	0	1	15
172		min	-668.934	3	-.03	3	-.017	2	0	3	0	3	-.001	4
173		11	max	777.515	2	.071	2	.041	3	0	1	0	1	15
174		min	-668.987	3	-.133	3	-.017	2	0	3	0	3	-.001	4
175		12	max	777.445	2	-.047	15	.041	3	0	1	0	1	15
176		min	-669.039	3	-.236	3	-.017	2	0	3	0	3	-.001	4
177		13	max	777.375	2	-.089	15	.041	3	0	1	0	1	15
178		min	-669.092	3	-.377	4	-.017	2	0	3	0	3	-.001	4
179		14	max	777.305	2	-.13	15	.041	3	0	1	0	1	15
180		min	-669.144	3	-.554	4	-.017	2	0	3	0	3	-.001	4
181		15	max	777.235	2	-.172	15	.041	3	0	1	0	1	15
182		min	-669.197	3	-.73	4	-.017	2	0	3	0	3	0	4
183		16	max	777.165	2	-.213	15	.041	3	0	1	0	1	15
184		min	-669.249	3	-.906	4	-.017	2	0	3	0	3	0	4
185		17	max	777.095	2	-.255	15	.041	3	0	1	0	1	15
186		min	-669.302	3	-1.083	4	-.017	2	0	3	0	3	0	4
187		18	max	777.025	2	-.296	15	.041	3	0	1	0	1	15
188		min	-669.354	3	-1.259	4	-.017	2	0	3	0	3	0	4
189		19	max	776.955	2	-.338	15	.041	3	0	1	0	1	1
190		min	-669.407	3	-1.435	4	-.017	2	0	3	0	3	0	1
191	M8	1	max	789.251	1	0	1	.476	1	0	1	0	10	1
192		min	27.29	15	0	1	-.653	3	0	1	0	3	0	1
193		2	max	789.316	1	0	1	.476	1	0	1	0	1	1
194		min	27.309	15	0	1	-.653	3	0	1	0	3	0	1
195		3	max	789.38	1	0	1	.476	1	0	1	0	1	1
196		min	27.329	15	0	1	-.653	3	0	1	0	3	0	1
197		4	max	789.445	1	0	1	.476	1	0	1	0	1	1
198		min	27.348	15	0	1	-.653	3	0	1	0	3	0	1
199		5	max	789.51	1	0	1	.476	1	0	1	0	1	1
200		min	27.368	15	0	1	-.653	3	0	1	0	3	0	1
201		6	max	789.575	1	0	1	.476	1	0	1	0	1	1
202		min	27.387	15	0	1	-.653	3	0	1	0	3	0	1
203		7	max	789.639	1	0	1	.476	1	0	1	0	1	1
204		min	27.407	15	0	1	-.653	3	0	1	0	3	0	1
205		8	max	789.704	1	0	1	.476	1	0	1	0	1	1
206		min	27.426	15	0	1	-.653	3	0	1	0	3	0	1
207		9	max	789.769	1	0	1	.476	1	0	1	0	1	1
208		min	27.446	15	0	1	-.653	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	789.833	1	0	1	.476	1	0	1	0	1	0	1
210		min	27.465	15	0	1	-.653	3	0	1	0	3	0	1
211	11	max	789.898	1	0	1	.476	1	0	1	0	1	0	1
212		min	27.485	15	0	1	-.653	3	0	1	0	3	0	1
213	12	max	789.963	1	0	1	.476	1	0	1	0	1	0	1
214		min	27.504	15	0	1	-.653	3	0	1	0	3	0	1
215	13	max	790.027	1	0	1	.476	1	0	1	0	1	0	1
216		min	27.524	15	0	1	-.653	3	0	1	0	3	0	1
217	14	max	790.092	1	0	1	.476	1	0	1	0	1	0	1
218		min	27.544	15	0	1	-.653	3	0	1	0	3	0	1
219	15	max	790.157	1	0	1	.476	1	0	1	0	1	0	1
220		min	27.563	15	0	1	-.653	3	0	1	0	3	0	1
221	16	max	790.222	1	0	1	.476	1	0	1	0	1	0	1
222		min	27.583	15	0	1	-.653	3	0	1	0	3	0	1
223	17	max	790.286	1	0	1	.476	1	0	1	0	1	0	1
224		min	27.602	15	0	1	-.653	3	0	1	0	3	0	1
225	18	max	790.351	1	0	1	.476	1	0	1	0	1	0	1
226		min	27.622	15	0	1	-.653	3	0	1	0	3	0	1
227	19	max	790.416	1	0	1	.476	1	0	1	0	1	0	1
228		min	27.641	15	0	1	-.653	3	0	1	-.001	3	0	1
229	M10	1	max	215.058	1	.675	.007	3	0	1	0	1	0	1
230		min	-318.376	3	.159	15	-.139	1	0	3	0	3	0	1
231	2	max	215.193	1	.617	4	.007	3	0	1	0	1	0	15
232		min	-318.274	3	.145	15	-.139	1	0	3	0	3	0	4
233	3	max	215.328	1	.56	4	.007	3	0	1	0	1	0	15
234		min	-318.173	3	.132	15	-.139	1	0	3	0	3	0	4
235	4	max	215.463	1	.502	4	.007	3	0	1	0	1	0	15
236		min	-318.072	3	.118	15	-.139	1	0	3	0	3	0	4
237	5	max	215.598	1	.445	4	.007	3	0	1	0	1	0	15
238		min	-317.971	3	.105	15	-.139	1	0	3	0	3	0	4
239	6	max	215.733	1	.387	4	.007	3	0	1	0	1	0	15
240		min	-317.87	3	.091	15	-.139	1	0	3	0	3	0	4
241	7	max	215.867	1	.33	4	.007	3	0	1	0	1	0	15
242		min	-317.769	3	.078	15	-.139	1	0	3	0	3	0	4
243	8	max	216.002	1	.272	4	.007	3	0	1	0	1	0	15
244		min	-317.668	3	.064	15	-.139	1	0	3	0	3	0	4
245	9	max	216.137	1	.215	4	.007	3	0	1	0	1	0	15
246		min	-317.566	3	.051	15	-.139	1	0	3	0	3	0	4
247	10	max	216.272	1	.157	4	.007	3	0	1	0	9	0	15
248		min	-317.465	3	.037	15	-.139	1	0	3	0	3	0	4
249	11	max	216.407	1	.11	2	.007	3	0	1	0	9	0	15
250		min	-317.364	3	.024	15	-.139	1	0	3	0	3	0	4
251	12	max	216.542	1	.065	2	.007	3	0	1	0	15	0	15
252		min	-317.263	3	.006	12	-.139	1	0	3	0	3	0	4
253	13	max	216.677	1	.02	2	.007	3	0	1	0	15	0	15
254		min	-317.162	3	-.027	3	-.139	1	0	3	0	3	0	4
255	14	max	216.811	1	-.017	15	.007	3	0	1	0	15	0	15
256		min	-317.061	3	-.073	4	-.139	1	0	3	0	3	0	4
257	15	max	216.946	1	-.03	15	.007	3	0	1	0	15	0	15
258		min	-316.96	3	-.13	4	-.139	1	0	3	0	3	0	4
259	16	max	217.081	1	-.044	15	.007	3	0	1	0	15	0	15
260		min	-316.858	3	-.188	4	-.139	1	0	3	0	3	0	4
261	17	max	217.216	1	-.057	15	.007	3	0	1	0	15	0	15
262		min	-316.757	3	-.245	4	-.139	1	0	3	0	3	0	4
263	18	max	217.351	1	-.071	15	.007	3	0	1	0	15	0	15
264		min	-316.656	3	-.303	4	-.139	1	0	3	0	1	0	4
265	19	max	217.486	1	-.084	15	.007	3	0	1	0	15	0	15

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-316.555	3	-.36	4	-.139	1	0	3	0	1	0	4
267	M11	1	max	219.995	2	1.738	4	.289	1	0	3	0	3	0	4
268			min	-219.887	3	.409	15	-.049	3	0	15	0	1	0	12
269		2	max	219.925	2	1.562	4	.289	1	0	3	0	3	0	2
270			min	-219.94	3	.367	15	-.049	3	0	15	0	1	0	3
271		3	max	219.855	2	1.386	4	.289	1	0	3	0	3	0	2
272			min	-219.992	3	.326	15	-.049	3	0	15	0	1	0	3
273		4	max	219.785	2	1.209	4	.289	1	0	3	0	3	0	15
274			min	-220.045	3	.284	15	-.049	3	0	15	0	1	0	3
275		5	max	219.715	2	1.033	4	.289	1	0	3	0	3	0	15
276			min	-220.097	3	.243	15	-.049	3	0	15	0	1	0	4
277		6	max	219.645	2	.856	4	.289	1	0	3	0	3	0	15
278			min	-220.15	3	.201	15	-.049	3	0	15	0	1	0	4
279		7	max	219.575	2	.68	4	.289	1	0	3	0	3	0	15
280			min	-220.202	3	.16	15	-.049	3	0	15	0	1	0	4
281		8	max	219.505	2	.504	4	.289	1	0	3	0	3	0	15
282			min	-220.255	3	.118	15	-.049	3	0	15	0	1	-.001	4
283		9	max	219.435	2	.327	4	.289	1	0	3	0	3	0	15
284			min	-220.307	3	.077	15	-.049	3	0	15	0	1	-.001	4
285		10	max	219.365	2	.151	4	.289	1	0	3	0	3	0	15
286			min	-220.36	3	.028	12	-.049	3	0	15	0	1	-.001	4
287		11	max	219.295	2	.005	2	.289	1	0	3	0	3	0	15
288			min	-220.412	3	-.067	3	-.049	3	0	15	0	1	-.001	4
289		12	max	219.225	2	-.047	15	.289	1	0	3	0	3	0	15
290			min	-220.465	3	-.202	4	-.049	3	0	15	0	1	-.001	4
291		13	max	219.155	2	-.089	15	.289	1	0	3	0	3	0	15
292			min	-220.517	3	-.378	4	-.049	3	0	15	0	1	-.001	4
293		14	max	219.085	2	-.13	15	.289	1	0	3	0	3	0	15
294			min	-220.57	3	-.554	4	-.049	3	0	15	0	1	-.001	4
295		15	max	219.015	2	-.172	15	.289	1	0	3	0	3	0	15
296			min	-220.622	3	-.731	4	-.049	3	0	15	0	1	0	4
297		16	max	218.945	2	-.213	15	.289	1	0	3	0	3	0	15
298			min	-220.675	3	-.907	4	-.049	3	0	15	0	2	0	4
299		17	max	218.875	2	-.255	15	.289	1	0	3	0	3	0	15
300			min	-220.727	3	-1.084	4	-.049	3	0	15	0	15	0	4
301		18	max	218.805	2	-.296	15	.289	1	0	3	0	3	0	15
302			min	-220.78	3	-1.26	4	-.049	3	0	15	0	15	0	4
303		19	max	218.735	2	-.338	15	.289	1	0	3	0	3	0	1
304			min	-220.832	3	-1.436	4	-.049	3	0	15	0	15	0	1
305	M12	1	max	304.771	1	0	1	1.869	1	0	1	0	2	0	1
306			min	12.33	15	0	1	.09	15	0	1	0	3	0	1
307		2	max	304.836	1	0	1	1.869	1	0	1	0	1	0	1
308			min	12.349	15	0	1	.09	15	0	1	0	15	0	1
309		3	max	304.9	1	0	1	1.869	1	0	1	0	1	0	1
310			min	12.369	15	0	1	.09	15	0	1	0	15	0	1
311		4	max	304.965	1	0	1	1.869	1	0	1	0	1	0	1
312			min	12.388	15	0	1	.09	15	0	1	0	15	0	1
313		5	max	305.03	1	0	1	1.869	1	0	1	0	1	0	1
314			min	12.408	15	0	1	.09	15	0	1	0	15	0	1
315		6	max	305.095	1	0	1	1.869	1	0	1	0	1	0	1
316			min	12.427	15	0	1	.09	15	0	1	0	15	0	1
317		7	max	305.159	1	0	1	1.869	1	0	1	.001	1	0	1
318			min	12.447	15	0	1	.09	15	0	1	0	15	0	1
319		8	max	305.224	1	0	1	1.869	1	0	1	.001	1	0	1
320			min	12.466	15	0	1	.09	15	0	1	0	15	0	1
321		9	max	305.289	1	0	1	1.869	1	0	1	.001	1	0	1
322			min	12.486	15	0	1	.09	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323		10	max	305.353	1	0	1	1.869	1	0	1	.002	1	0	1
324			min	12.506	15	0	1	.09	15	0	1	0	15	0	1
325		11	max	305.418	1	0	1	1.869	1	0	1	.002	1	0	1
326			min	12.525	15	0	1	.09	15	0	1	0	15	0	1
327		12	max	305.483	1	0	1	1.869	1	0	1	.002	1	0	1
328			min	12.545	15	0	1	.09	15	0	1	0	15	0	1
329		13	max	305.548	1	0	1	1.869	1	0	1	.002	1	0	1
330			min	12.564	15	0	1	.09	15	0	1	0	15	0	1
331		14	max	305.612	1	0	1	1.869	1	0	1	.002	1	0	1
332			min	12.584	15	0	1	.09	15	0	1	0	15	0	1
333		15	max	305.677	1	0	1	1.869	1	0	1	.002	1	0	1
334			min	12.603	15	0	1	.09	15	0	1	0	15	0	1
335		16	max	305.742	1	0	1	1.869	1	0	1	.003	1	0	1
336			min	12.623	15	0	1	.09	15	0	1	0	15	0	1
337		17	max	305.806	1	0	1	1.869	1	0	1	.003	1	0	1
338			min	12.642	15	0	1	.09	15	0	1	0	15	0	1
339		18	max	305.871	1	0	1	1.869	1	0	1	.003	1	0	1
340			min	12.662	15	0	1	.09	15	0	1	0	15	0	1
341		19	max	305.936	1	0	1	1.869	1	0	1	.003	1	0	1
342			min	12.681	15	0	1	.09	15	0	1	0	15	0	1
343	M1	1	max	106.242	1	346.194	3	-1.784	15	0	2	.074	1	0	2
344			min	4.855	15	-229.612	2	-37.758	1	0	3	.003	15	0	3
345		2	max	106.402	1	346.023	3	-1.784	15	0	2	.066	1	.05	2
346			min	4.904	15	-229.841	2	-37.758	1	0	3	.003	15	-.075	3
347		3	max	117.421	3	5.243	9	-1.773	15	0	12	.057	1	.099	2
348			min	-18.617	10	-29.625	2	-37.666	1	0	1	.003	15	-.149	3
349		4	max	117.541	3	5.053	9	-1.773	15	0	12	.049	1	.106	2
350			min	-18.484	10	-29.854	2	-37.666	1	0	1	.002	15	-.147	3
351		5	max	117.661	3	4.862	9	-1.773	15	0	12	.041	1	.112	2
352			min	-18.35	10	-30.083	2	-37.666	1	0	1	.002	15	-.145	3
353		6	max	117.781	3	4.671	9	-1.773	15	0	12	.033	1	.119	2
354			min	-18.217	10	-30.311	2	-37.666	1	0	1	.002	15	-.143	3
355		7	max	117.901	3	4.481	9	-1.773	15	0	12	.025	1	.125	2
356			min	-18.083	10	-30.54	2	-37.666	1	0	1	.001	15	-.141	3
357		8	max	118.021	3	4.29	9	-1.773	15	0	12	.016	1	.132	2
358			min	-17.95	10	-30.769	2	-37.666	1	0	1	0	15	-.139	3
359		9	max	118.141	3	4.099	9	-1.773	15	0	12	.008	1	.139	2
360			min	-17.816	10	-30.997	2	-37.666	1	0	1	0	15	-.137	3
361		10	max	118.261	3	3.909	9	-1.773	15	0	12	.002	3	.145	2
362			min	-17.683	10	-31.226	2	-37.666	1	0	1	0	10	-.134	3
363		11	max	118.382	3	3.718	9	-1.773	15	0	12	0	3	.152	2
364			min	-17.549	10	-31.455	2	-37.666	1	0	1	-.008	1	-.132	3
365		12	max	118.502	3	3.528	9	-1.773	15	0	12	0	12	.159	2
366			min	-17.416	10	-31.684	2	-37.666	1	0	1	-.016	1	-.13	3
367		13	max	118.622	3	3.337	9	-1.773	15	0	12	-.001	15	.166	2
368			min	-17.282	10	-31.912	2	-37.666	1	0	1	-.024	1	-.128	3
369		14	max	118.742	3	3.146	9	-1.773	15	0	12	-.002	15	.173	2
370			min	-17.149	10	-32.141	2	-37.666	1	0	1	-.033	1	-.125	3
371		15	max	118.862	3	2.956	9	-1.773	15	0	12	-.002	15	.18	2
372			min	-17.015	10	-32.37	2	-37.666	1	0	1	-.041	1	-.123	3
373		16	max	89.06	2	160.783	2	-1.786	15	0	1	-.002	15	.185	2
374			min	2.004	15	-205.948	3	-37.893	1	0	12	-.049	1	-.119	3
375		17	max	89.22	2	160.554	2	-1.786	15	0	1	-.003	15	.15	2
376			min	2.052	15	-206.12	3	-37.893	1	0	12	-.057	1	-.074	3
377		18	max	-4.901	15	347.542	2	-1.827	15	0	3	-.003	15	.076	2
378			min	-106.392	1	-170.367	3	-38.858	1	0	2	-.066	1	-.037	3
379		19	max	-4.853	15	347.313	2	-1.827	15	0	3	-.003	15	0	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-106.232	1	-170.539	3	-38.858	1	0	2	-.074	1	0	3
381	M5	max	243.958	1	1129.822	3	0	2	0	1	.009	3	0	3
382		min	6.228	12	-745.183	2	-68.269	3	0	3	0	2	0	2
383		max	244.118	1	1129.651	3	0	2	0	1	0	1	.161	2
384		min	6.308	12	-745.412	2	-68.269	3	0	3	-.006	3	-.245	3
385		max	356.74	3	4.883	9	7.661	3	0	3	0	1	.32	2
386		min	-84.892	2	-104.369	2	-.525	1	0	1	-.02	3	-.484	3
387		max	356.861	3	4.693	9	7.661	3	0	3	0	1	.343	2
388		min	-84.732	2	-104.597	2	-.525	1	0	1	-.019	3	-.477	3
389		max	356.981	3	4.502	9	7.661	3	0	3	0	1	.366	2
390		min	-84.572	2	-104.826	2	-.525	1	0	1	-.017	3	-.469	3
391		max	357.101	3	4.311	9	7.661	3	0	3	0	1	.388	2
392		min	-84.411	2	-105.055	2	-.525	1	0	1	-.015	3	-.461	3
393		max	357.221	3	4.121	9	7.661	3	0	3	0	1	.411	2
394		min	-84.251	2	-105.284	2	-.525	1	0	1	-.014	3	-.453	3
395		max	357.341	3	3.93	9	7.661	3	0	3	0	1	.434	2
396		min	-84.091	2	-105.512	2	-.525	1	0	1	-.012	3	-.445	3
397		max	357.461	3	3.739	9	7.661	3	0	3	0	1	.457	2
398		min	-83.931	2	-105.741	2	-.525	1	0	1	-.01	3	-.437	3
399		max	357.581	3	3.549	9	7.661	3	0	3	0	2	.48	2
400		min	-83.771	2	-105.97	2	-.525	1	0	1	-.009	3	-.429	3
401		max	357.701	3	3.358	9	7.661	3	0	3	0	2	.503	2
402		min	-83.611	2	-106.199	2	-.525	1	0	1	-.007	3	-.421	3
403		max	357.822	3	3.168	9	7.661	3	0	3	0	2	.526	2
404		min	-83.45	2	-106.427	2	-.525	1	0	1	-.005	3	-.413	3
405		max	357.942	3	2.977	9	7.661	3	0	3	0	10	.549	2
406		min	-83.29	2	-106.656	2	-.525	1	0	1	-.004	3	-.404	3
407		max	358.062	3	2.786	9	7.661	3	0	3	0	10	.572	2
408		min	-83.13	2	-106.885	2	-.525	1	0	1	-.002	3	-.396	3
409		max	358.182	3	2.596	9	7.661	3	0	3	0	10	.596	2
410		min	-82.97	2	-107.114	2	-.525	1	0	1	0	1	-.388	3
411		max	282.484	2	581.829	2	7.641	3	0	3	0	3	.613	2
412		min	4.607	15	-630.278	3	-.546	1	0	10	0	1	-.374	3
413		max	282.644	2	581.601	2	7.641	3	0	3	.003	3	.487	2
414		min	4.655	15	-630.449	3	-.546	1	0	10	0	1	-.238	3
415		max	-8.552	12	1128.189	2	6.982	3	0	10	.004	3	.244	2
416		min	-244.133	1	-548.148	3	-.12	1	0	1	0	1	-.118	3
417		max	-8.472	12	1127.96	2	6.982	3	0	10	.006	3	0	3
418		min	-243.973	1	-548.319	3	-.12	1	0	1	0	1	0	2
419	M9	max	105.891	1	346.126	3	73.648	3	0	3	-.003	15	0	2
420		min	4.835	15	-229.612	2	1.852	15	0	2	-.073	1	0	3
421		max	106.051	1	345.955	3	73.648	3	0	3	-.001	12	.05	2
422		min	4.883	15	-229.841	2	1.852	15	0	2	-.065	1	-.075	3
423		max	117.243	3	5.224	9	36.844	1	0	1	.013	3	.099	2
424		min	-18.158	10	-29.629	2	-1.593	3	0	12	-.056	1	-.149	3
425		max	117.363	3	5.033	9	36.844	1	0	1	.013	3	.105	2
426		min	-18.024	10	-29.858	2	-1.593	3	0	12	-.048	1	-.147	3
427		max	117.483	3	4.843	9	36.844	1	0	1	.013	3	.112	2
428		min	-17.891	10	-30.086	2	-1.593	3	0	12	-.04	1	-.145	3
429		max	117.603	3	4.652	9	36.844	1	0	1	.012	3	.119	2
430		min	-17.757	10	-30.315	2	-1.593	3	0	12	-.032	1	-.143	3
431		max	117.723	3	4.462	9	36.844	1	0	1	.012	3	.125	2
432		min	-17.624	10	-30.544	2	-1.593	3	0	12	-.024	1	-.141	3
433		max	117.844	3	4.271	9	36.844	1	0	1	.011	3	.132	2
434		min	-17.49	10	-30.772	2	-1.593	3	0	12	-.016	1	-.139	3
435		max	117.964	3	4.08	9	36.844	1	0	1	.011	3	.138	2
436		min	-17.357	10	-31.001	2	-1.593	3	0	12	-.008	1	-.137	3





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494			min	1.784	15	-229.478	2	4.855	15	0	2	.003	15	0	3
495	M16	1	max	1.119	3	347.486	2	-4.834	15	0	3	.073	1	0	2
496			min	-38.889	1	-170.562	3	-105.897	1	0	2	.003	15	0	3
497		2	max	1.119	3	246.124	2	-3.691	15	0	3	.019	1	.085	3
498			min	-38.889	1	-121.141	3	-80.559	1	0	2	0	10	-.173	2
499		3	max	1.119	3	144.762	2	-2.547	15	0	3	0	12	.141	3
500			min	-38.889	1	-71.72	3	-55.22	1	0	2	-.021	1	-.287	2
501		4	max	1.119	3	43.401	2	-1.403	15	0	3	-.002	15	.169	3
502			min	-38.889	1	-22.299	3	-29.882	1	0	2	-.046	1	-.342	2
503		5	max	1.119	3	27.122	3	.74	10	0	3	-.003	15	.167	3
504			min	-38.889	1	-57.961	2	-4.543	1	0	2	-.056	1	-.338	2
505		6	max	1.119	3	76.543	3	20.795	1	0	3	-.002	15	.137	3
506			min	-38.889	1	-159.323	2	-.925	3	0	2	-.051	1	-.274	2
507		7	max	1.119	3	125.964	3	46.134	1	0	3	-.001	15	.078	3
508			min	-38.889	1	-260.685	2	.641	12	0	2	-.031	1	-.152	2
509		8	max	1.119	3	175.385	3	71.472	1	0	3	.004	2	.03	2
510			min	-38.889	1	-362.046	2	1.75	12	0	2	-.006	3	-.01	3
511		9	max	1.119	3	224.806	3	96.811	1	0	3	.052	1	.271	2
512			min	-38.889	1	-463.408	2	2.86	12	0	2	-.004	3	-.127	3
513		10	max	-1.828	15	-9.111	15	122.149	1	0	15	.116	1	.57	2
514			min	-38.889	1	-564.77	2	-6.627	3	0	2	.003	12	-.272	3
515		11	max	-1.827	15	463.408	2	-3.393	12	0	2	.051	1	.271	2
516			min	-38.764	1	-224.806	3	-96.467	1	0	3	0	12	-.127	3
517		12	max	-1.827	15	362.046	2	-2.284	12	0	2	.004	2	.03	2
518			min	-38.764	1	-175.385	3	-71.128	1	0	3	-.001	3	-.01	3
519		13	max	-1.827	15	260.685	2	-1.174	12	0	2	-.001	15	.078	3
520			min	-38.764	1	-125.964	3	-45.79	1	0	3	-.032	1	-.152	2
521		14	max	-1.827	15	159.323	2	.031	3	0	2	-.002	12	.137	3
522			min	-38.764	1	-76.543	3	-20.451	1	0	3	-.051	1	-.274	2
523		15	max	-1.827	15	57.961	2	4.887	1	0	2	-.002	12	.167	3
524			min	-38.764	1	-27.122	3	-.74	10	0	3	-.055	1	-.338	2
525		16	max	-1.827	15	22.299	3	30.226	1	0	2	0	12	.169	3
526			min	-38.764	1	-43.401	2	1.422	15	0	3	-.045	1	-.342	2
527		17	max	-1.827	15	71.72	3	55.564	1	0	2	.001	3	.141	3
528			min	-38.764	1	-144.762	2	2.565	15	0	3	-.02	1	-.287	2
529		18	max	-1.827	15	121.141	3	80.903	1	0	2	.02	1	.085	3
530			min	-38.764	1	-246.124	2	3.709	15	0	3	0	10	-.173	2
531		19	max	-1.827	15	170.562	3	106.241	1	0	2	.074	1	0	2
532			min	-38.764	1	-347.486	2	4.853	15	0	3	.003	15	0	3
533	M15	1	max	.771	13	1.86	4	.086	3	0	9	0	9	0	1
534			min	-91.278	3	0	1	-.019	9	0	3	0	3	0	1
535		2	max	.668	13	1.653	4	.086	3	0	9	0	9	0	1
536			min	-91.354	3	0	1	-.019	9	0	3	0	3	0	4
537		3	max	.564	13	1.446	4	.086	3	0	9	0	9	0	1
538			min	-91.429	3	0	1	-.019	9	0	3	0	3	-.001	4
539		4	max	.46	13	1.24	4	.086	3	0	9	0	9	0	1
540			min	-91.505	3	0	1	-.019	9	0	3	0	3	-.002	4
541		5	max	.356	13	1.033	4	.086	3	0	9	0	9	0	1
542			min	-91.58	3	0	1	-.019	9	0	3	0	3	-.002	4
543		6	max	.252	13	.827	4	.086	3	0	9	0	9	0	1
544			min	-91.656	3	0	1	-.019	9	0	3	0	3	-.002	4
545		7	max	.148	13	.62	4	.086	3	0	9	0	3	0	1
546			min	-91.731	3	0	1	-.019	9	0	3	0	9	-.003	4
547		8	max	.044	13	.413	4	.086	3	0	9	0	3	0	1
548			min	-91.807	3	0	1	-.019	9	0	3	0	9	-.003	4
549		9	max	0	1	.207	4	.086	3	0	9	0	3	0	1
550			min	-91.882	3	0	1	-.019	9	0	3	0	9	-.003	4





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-90.052	3	-1.86	4	-.035	3	0	2	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.002	1	.011	2	.007	1	-2.891e-5	15	NC	3	NC	2	
2			min	-.004	3	-.011	3	-.002	3	-6.159e-4	1	3958.882	2	5786.387	1	
3			2	max	.002	1	.01	2	.007	1	-2.764e-5	15	NC	3	NC	2
4				min	-.004	3	-.011	3	-.002	3	-5.888e-4	1	4330.489	2	6212.26	1
5			3	max	.002	1	.009	2	.006	1	-2.637e-5	15	NC	3	NC	2
6				min	-.003	3	-.01	3	-.002	3	-5.616e-4	1	4774.244	2	6716.88	1
7			4	max	.002	1	.008	2	.006	1	-2.51e-5	15	NC	1	NC	2
8				min	-.003	3	-.01	3	-.001	3	-5.344e-4	1	5307.803	2	7318.769	1
9			5	max	.002	1	.007	2	.005	1	-2.383e-5	15	NC	1	NC	2
10				min	-.003	3	-.009	3	-.001	3	-5.072e-4	1	5954.771	2	8042.676	1
11		6	max	.002	1	.006	2	.005	1	-2.257e-5	15	NC	1	NC	2	
12			min	-.003	3	-.009	3	-.001	3	-4.8e-4	1	6747.208	2	8922.238	1	
13		7	max	.001	1	.005	2	.004	1	-2.13e-5	15	NC	1	NC	1	
14			min	-.003	3	-.008	3	0	3	-4.528e-4	1	7729.482	2	NC	1	
15		8	max	.001	1	.005	2	.004	1	-2.003e-5	15	NC	1	NC	1	
16			min	-.002	3	-.008	3	0	3	-4.257e-4	1	8964.365	2	NC	1	
17		9	max	.001	1	.004	2	.003	1	-1.876e-5	15	NC	1	NC	1	
18			min	-.002	3	-.007	3	0	3	-3.985e-4	1	NC	1	NC	1	
19		10	max	.001	1	.003	2	.003	1	-1.75e-5	15	NC	1	NC	1	
20			min	-.002	3	-.007	3	0	3	-3.713e-4	1	NC	1	NC	1	
21		11	max	0	1	.003	2	.002	1	-1.623e-5	15	NC	1	NC	1	
22			min	-.002	3	-.006	3	0	3	-3.441e-4	1	NC	1	NC	1	
23		12	max	0	1	.002	2	.002	1	-1.496e-5	15	NC	1	NC	1	
24			min	-.002	3	-.005	3	0	3	-3.169e-4	1	NC	1	NC	1	
25		13	max	0	1	.002	2	.002	1	-1.369e-5	15	NC	1	NC	1	
26			min	-.001	3	-.005	3	0	3	-2.897e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.001	1	-1.242e-5	15	NC	1	NC	1	
28			min	-.001	3	-.004	3	0	3	-2.626e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	0	1	-1.116e-5	15	NC	1	NC	1	
30			min	0	3	-.003	3	0	3	-2.354e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-9.888e-6	15	NC	1	NC	1	
32			min	0	3	-.002	3	0	3	-2.082e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-8.62e-6	15	NC	1	NC	1	
34			min	0	3	-.002	3	0	3	-1.81e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-7.353e-6	15	NC	1	NC	1	
36			min	0	3	0	3	0	3	-1.538e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-5.807e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-1.267e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	6.066e-5	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	2.913e-6	15	NC	1	NC	1	
41			2	max	0	3	0	2	0	12	7.401e-5	1	NC	1	NC	1
42				min	0	2	0	3	0	1	3.52e-6	15	NC	1	NC	1
43			3	max	0	3	0	2	0	12	8.737e-5	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	4.126e-6	15	NC	1	NC	1
45			4	max	0	3	0	2	0	3	1.007e-4	1	NC	1	NC	1
46				min	0	2	-.003	3	0	1	4.733e-6	15	NC	1	NC	1
47			5	max	0	3	0	2	0	3	1.141e-4	1	NC	1	NC	1
48				min	0	2	-.004	3	0	1	5.339e-6	15	NC	1	NC	1
49			6	max	0	3	0	2	0	3	1.274e-4	1	NC	1	NC	1
50				min	0	2	-.005	3	0	1	5.946e-6	15	NC	1	NC	1
51		7	max	0	3	0	2	0	3	1.408e-4	1	NC	1	NC	1	



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52			min	0	2	-.005	3	0	1	6.553e-6	15	NC	1	NC	1
53		8	max	0	3	.001	2	0	3	1.541e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	7.159e-6	15	NC	1	NC	1
55		9	max	.001	3	.001	2	0	3	1.675e-4	1	NC	1	NC	1
56			min	-.001	2	-.007	3	0	9	7.766e-6	15	NC	1	NC	1
57		10	max	.001	3	.002	2	0	2	1.809e-4	1	NC	1	NC	1
58			min	-.001	2	-.007	3	0	15	8.372e-6	15	NC	1	NC	1
59		11	max	.001	3	.002	2	0	1	1.942e-4	1	NC	1	NC	1
60			min	-.001	2	-.008	3	0	15	8.979e-6	15	NC	1	NC	1
61		12	max	.002	3	.003	2	0	1	2.076e-4	1	NC	1	NC	1
62			min	-.002	2	-.008	3	0	15	9.585e-6	15	NC	1	NC	1
63		13	max	.002	3	.004	2	.001	1	2.209e-4	1	NC	1	NC	1
64			min	-.002	2	-.008	3	0	15	1.019e-5	15	NC	1	NC	1
65		14	max	.002	3	.005	2	.002	1	2.343e-4	1	NC	1	NC	1
66			min	-.002	2	-.009	3	0	15	1.08e-5	15	NC	1	NC	1
67		15	max	.002	3	.005	2	.002	1	2.476e-4	1	NC	1	NC	1
68			min	-.002	2	-.009	3	0	15	1.14e-5	15	8523.619	2	NC	1
69		16	max	.002	3	.006	2	.003	1	2.61e-4	1	NC	1	NC	1
70			min	-.002	2	-.009	3	0	15	1.201e-5	15	7235.804	2	NC	1
71		17	max	.002	3	.007	2	.003	1	2.743e-4	1	NC	1	NC	1
72			min	-.002	2	-.009	3	0	15	1.262e-5	15	6237.12	2	NC	1
73		18	max	.002	3	.008	2	.003	1	2.877e-4	1	NC	1	NC	1
74			min	-.002	2	-.009	3	0	15	1.322e-5	15	5453.925	2	NC	1
75		19	max	.002	3	.01	2	.004	1	3.01e-4	1	NC	3	NC	1
76			min	-.002	2	-.009	3	0	15	1.383e-5	15	4834.303	2	NC	1
77	M4	1	max	.001	1	.013	2	0	15	-1.746e-5	12	NC	1	NC	2
78			min	0	15	-.011	3	-.003	1	-4.832e-4	1	NC	1	6701.921	1
79		2	max	.001	1	.012	2	0	15	-1.746e-5	12	NC	1	NC	2
80			min	0	15	-.01	3	-.003	1	-4.832e-4	1	NC	1	7311.371	1
81		3	max	.001	1	.011	2	0	15	-1.746e-5	12	NC	1	NC	2
82			min	0	15	-.01	3	-.002	1	-4.832e-4	1	NC	1	8036.706	1
83		4	max	.001	1	.01	2	0	15	-1.746e-5	12	NC	1	NC	2
84			min	0	15	-.009	3	-.002	1	-4.832e-4	1	NC	1	8908.493	1
85		5	max	.001	1	.01	2	0	15	-1.746e-5	12	NC	1	NC	2
86			min	0	15	-.009	3	-.002	1	-4.832e-4	1	NC	1	9968.375	1
87		6	max	.001	1	.009	2	0	15	-1.746e-5	12	NC	1	NC	1
88			min	0	15	-.008	3	-.002	1	-4.832e-4	1	NC	1	NC	1
89		7	max	0	1	.008	2	0	15	-1.746e-5	12	NC	1	NC	1
90			min	0	15	-.007	3	-.001	1	-4.832e-4	1	NC	1	NC	1
91		8	max	0	1	.008	2	0	15	-1.746e-5	12	NC	1	NC	1
92			min	0	15	-.007	3	-.001	1	-4.832e-4	1	NC	1	NC	1
93		9	max	0	1	.007	2	0	15	-1.746e-5	12	NC	1	NC	1
94			min	0	15	-.006	3	-.001	1	-4.832e-4	1	NC	1	NC	1
95		10	max	0	1	.006	2	0	15	-1.746e-5	12	NC	1	NC	1
96			min	0	15	-.006	3	0	1	-4.832e-4	1	NC	1	NC	1
97		11	max	0	1	.006	2	0	15	-1.746e-5	12	NC	1	NC	1
98			min	0	15	-.005	3	0	1	-4.832e-4	1	NC	1	NC	1
99		12	max	0	1	.005	2	0	15	-1.746e-5	12	NC	1	NC	1
100			min	0	15	-.004	3	0	1	-4.832e-4	1	NC	1	NC	1
101		13	max	0	1	.004	2	0	15	-1.746e-5	12	NC	1	NC	1
102			min	0	15	-.004	3	0	1	-4.832e-4	1	NC	1	NC	1
103		14	max	0	1	.003	2	0	15	-1.746e-5	12	NC	1	NC	1
104			min	0	15	-.003	3	0	1	-4.832e-4	1	NC	1	NC	1
105		15	max	0	1	.003	2	0	15	-1.746e-5	12	NC	1	NC	1
106			min	0	15	-.002	3	0	1	-4.832e-4	1	NC	1	NC	1
107		16	max	0	1	.002	2	0	15	-1.746e-5	12	NC	1	NC	1
108			min	0	15	-.002	3	0	1	-4.832e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	.001	2	0	15	-1.746e-5	12	NC	1	NC	1
110			min	0	15	-.001	3	0	1	-4.832e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-1.746e-5	12	NC	1	NC	1
112			min	0	15	0	3	0	1	-4.832e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-1.746e-5	12	NC	1	NC	1
114			min	0	1	0	1	0	1	-4.832e-4	1	NC	1	NC	1
115	M6	1	max	.007	1	.036	2	.003	1	4.743e-4	3	NC	3	NC	1
116			min	-.012	3	-.035	3	-.006	3	-2.028e-7	2	1163.36	2	7106.513	3
117		2	max	.007	1	.034	2	.003	1	4.586e-4	3	NC	3	NC	1
118			min	-.011	3	-.033	3	-.006	3	-1.694e-6	1	1245.767	2	7516.469	3
119		3	max	.006	1	.032	2	.002	1	4.429e-4	3	NC	3	NC	1
120			min	-.011	3	-.031	3	-.005	3	-4.88e-6	1	1340.309	2	8006.948	3
121		4	max	.006	1	.029	2	.002	1	4.273e-4	3	NC	3	NC	1
122			min	-.01	3	-.029	3	-.005	3	-8.067e-6	1	1449.406	2	8594.546	3
123		5	max	.006	1	.027	2	.002	1	4.116e-4	3	NC	3	NC	1
124			min	-.009	3	-.028	3	-.005	3	-1.125e-5	1	1576.177	2	9301.366	3
125		6	max	.005	1	.025	2	.002	1	3.959e-4	3	NC	3	NC	1
126			min	-.009	3	-.026	3	-.004	3	-1.444e-5	1	1724.703	2	NC	1
127		7	max	.005	1	.022	2	.002	1	3.802e-4	3	NC	3	NC	1
128			min	-.008	3	-.024	3	-.004	3	-1.763e-5	1	1900.426	2	NC	1
129		8	max	.004	1	.02	2	.001	1	3.646e-4	3	NC	3	NC	1
130			min	-.007	3	-.022	3	-.003	3	-2.081e-5	1	2110.766	2	NC	1
131		9	max	.004	1	.018	2	.001	1	3.489e-4	3	NC	3	NC	1
132			min	-.007	3	-.02	3	-.003	3	-2.4e-5	1	2366.107	2	NC	1
133		10	max	.004	1	.016	2	.001	1	3.332e-4	3	NC	3	NC	1
134			min	-.006	3	-.018	3	-.003	3	-2.719e-5	1	2681.442	2	NC	1
135		11	max	.003	1	.014	2	0	1	3.176e-4	3	NC	3	NC	1
136			min	-.005	3	-.016	3	-.002	3	-3.037e-5	1	3079.24	2	NC	1
137		12	max	.003	1	.012	2	0	1	3.019e-4	3	NC	3	NC	1
138			min	-.005	3	-.014	3	-.002	3	-3.356e-5	1	3594.792	2	NC	1
139		13	max	.002	1	.01	2	0	1	2.862e-4	3	NC	3	NC	1
140			min	-.004	3	-.012	3	-.002	3	-3.675e-5	1	4286.875	2	NC	1
141		14	max	.002	1	.008	2	0	1	2.705e-4	3	NC	3	NC	1
142			min	-.003	3	-.01	3	-.001	3	-3.993e-5	1	5261.237	2	NC	1
143		15	max	.002	1	.006	2	0	1	2.549e-4	3	NC	1	NC	1
144			min	-.003	3	-.008	3	0	3	-4.312e-5	1	6729.282	2	NC	1
145		16	max	.001	1	.005	2	0	1	2.392e-4	3	NC	1	NC	1
146			min	-.002	3	-.006	3	0	3	-4.631e-5	1	9184.155	2	NC	1
147		17	max	0	1	.003	2	0	1	2.235e-4	3	NC	1	NC	1
148			min	-.001	3	-.004	3	0	3	-4.949e-5	1	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	2.078e-4	3	NC	1	NC	1
150			min	0	3	-.002	3	0	3	-5.268e-5	1	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.922e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-5.587e-5	1	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.655e-5	1	NC	1	NC	1
154			min	0	1	0	1	0	1	-9.153e-5	3	NC	1	NC	1
155		2	max	0	3	.002	2	0	3	2.269e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-6.76e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.883e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	1	-4.367e-5	3	NC	1	NC	1
159		4	max	.001	3	.004	2	.001	3	1.497e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	1	-1.975e-5	3	NC	1	NC	1
161		5	max	.002	3	.006	2	.002	3	1.112e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	1	0	10	7835.427	2	NC	1
163		6	max	.002	3	.007	2	.002	3	2.81e-5	3	NC	1	NC	1
164			min	-.002	2	-.01	3	0	1	0	10	6273.529	2	NC	1
165		7	max	.003	3	.009	2	.002	3	5.203e-5	3	NC	3	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.003	2	-.012	3	0	1	-1.938e-7	13	5208.433	2	NC	1
167		8	max	.003	3	.01	2	.002	3	7.595e-5	3	NC	3	NC	1
168			min	-.003	2	-.014	3	0	1	-1.043e-6	4	4429.897	2	NC	1
169		9	max	.003	3	.012	2	.003	3	9.988e-5	3	NC	3	NC	1
170			min	-.004	2	-.016	3	0	1	-4.315e-6	1	3833.073	2	NC	1
171		10	max	.004	3	.014	2	.003	3	1.238e-4	3	NC	3	NC	1
172			min	-.004	2	-.017	3	0	1	-8.173e-6	1	3359.805	2	NC	1
173		11	max	.004	3	.015	2	.003	3	1.477e-4	3	NC	3	NC	1
174			min	-.005	2	-.019	3	-.001	1	-1.203e-5	1	2975.16	2	NC	1
175		12	max	.005	3	.017	2	.003	3	1.717e-4	3	NC	3	NC	1
176			min	-.005	2	-.02	3	-.001	1	-1.589e-5	1	2656.797	2	NC	1
177		13	max	.005	3	.019	2	.003	3	1.956e-4	3	NC	3	NC	1
178			min	-.006	2	-.022	3	-.001	1	-1.975e-5	1	2389.673	2	NC	1
179		14	max	.005	3	.021	2	.003	3	2.195e-4	3	NC	3	NC	1
180			min	-.006	2	-.023	3	-.001	1	-2.36e-5	1	2163.211	2	NC	1
181		15	max	.006	3	.023	2	.003	3	2.434e-4	3	NC	3	NC	1
182			min	-.007	2	-.024	3	-.001	1	-2.746e-5	1	1969.707	2	NC	1
183		16	max	.006	3	.026	2	.003	3	2.674e-4	3	NC	3	NC	1
184			min	-.007	2	-.025	3	-.001	1	-3.132e-5	1	1803.373	2	NC	1
185		17	max	.007	3	.028	2	.003	3	2.913e-4	3	NC	3	NC	1
186			min	-.008	2	-.026	3	-.001	1	-3.518e-5	1	1659.755	2	NC	1
187		18	max	.007	3	.03	2	.003	3	3.152e-4	3	NC	3	NC	1
188			min	-.008	2	-.027	3	-.002	1	-3.904e-5	1	1535.359	2	NC	1
189		19	max	.008	3	.032	2	.003	3	3.391e-4	3	NC	3	NC	1
190			min	-.009	2	-.028	3	-.002	1	-4.289e-5	1	1427.399	2	NC	1
191	M8	1	max	.004	1	.042	2	.002	1	-1.104e-7	10	NC	1	NC	1
192			min	0	15	-.035	3	-.002	3	-2.617e-4	3	NC	1	9368.236	3
193		2	max	.004	1	.04	2	.001	1	-1.104e-7	10	NC	1	NC	1
194			min	0	15	-.033	3	-.002	3	-2.617e-4	3	NC	1	NC	1
195		3	max	.003	1	.038	2	.001	1	-1.104e-7	10	NC	1	NC	1
196			min	0	15	-.031	3	-.002	3	-2.617e-4	3	NC	1	NC	1
197		4	max	.003	1	.035	2	.001	1	-1.104e-7	10	NC	1	NC	1
198			min	0	15	-.029	3	-.002	3	-2.617e-4	3	NC	1	NC	1
199		5	max	.003	1	.033	2	.001	1	-1.104e-7	10	NC	1	NC	1
200			min	0	15	-.027	3	-.001	3	-2.617e-4	3	NC	1	NC	1
201		6	max	.003	1	.03	2	0	1	-1.104e-7	10	NC	1	NC	1
202			min	0	15	-.025	3	-.001	3	-2.617e-4	3	NC	1	NC	1
203		7	max	.003	1	.028	2	0	1	-1.104e-7	10	NC	1	NC	1
204			min	0	15	-.023	3	-.001	3	-2.617e-4	3	NC	1	NC	1
205		8	max	.002	1	.026	2	0	1	-1.104e-7	10	NC	1	NC	1
206			min	0	15	-.021	3	0	3	-2.617e-4	3	NC	1	NC	1
207		9	max	.002	1	.023	2	0	1	-1.104e-7	10	NC	1	NC	1
208			min	0	15	-.019	3	0	3	-2.617e-4	3	NC	1	NC	1
209		10	max	.002	1	.021	2	0	1	-1.104e-7	10	NC	1	NC	1
210			min	0	15	-.017	3	0	3	-2.617e-4	3	NC	1	NC	1
211		11	max	.002	1	.019	2	0	1	-1.104e-7	10	NC	1	NC	1
212			min	0	15	-.015	3	0	3	-2.617e-4	3	NC	1	NC	1
213		12	max	.001	1	.016	2	0	1	-1.104e-7	10	NC	1	NC	1
214			min	0	15	-.013	3	0	3	-2.617e-4	3	NC	1	NC	1
215		13	max	.001	1	.014	2	0	1	-1.104e-7	10	NC	1	NC	1
216			min	0	15	-.012	3	0	3	-2.617e-4	3	NC	1	NC	1
217		14	max	.001	1	.012	2	0	1	-1.104e-7	10	NC	1	NC	1
218			min	0	15	-.01	3	0	3	-2.617e-4	3	NC	1	NC	1
219		15	max	0	1	.009	2	0	1	-1.104e-7	10	NC	1	NC	1
220			min	0	15	-.008	3	0	3	-2.617e-4	3	NC	1	NC	1
221		16	max	0	1	.007	2	0	1	-1.104e-7	10	NC	1	NC	1
222			min	0	15	-.006	3	0	3	-2.617e-4	3	NC	1	NC	1







Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	.001	2	0	1	3.904e-4	1	NC	1	NC	1
338			min	0	15	-.001	3	0	15	1.863e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	3.904e-4	1	NC	1	NC	1
340			min	0	15	0	3	0	15	1.863e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.904e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	1.863e-5	15	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.003	3	9.429e-3	2	NC	1	NC	1
344			min	-.01	2	-.023	2	-.003	1	-1.382e-2	3	NC	1	NC	1
345		2	max	.01	3	.017	3	.003	3	4.62e-3	2	NC	4	NC	1
346			min	-.01	2	-.014	2	-.006	1	-6.834e-3	3	5117.376	2	NC	1
347		3	max	.01	3	.007	3	.002	3	1.924e-5	3	NC	4	NC	2
348			min	-.01	2	-.005	2	-.007	1	-3.767e-4	1	2625.197	2	9889.011	1
349		4	max	.01	3	.003	2	.002	3	2.263e-5	3	NC	4	NC	2
350			min	-.01	2	-.002	3	-.008	1	-3.258e-4	1	1834.315	2	8203.18	1
351		5	max	.01	3	.009	2	.001	3	2.602e-5	3	NC	4	NC	2
352			min	-.01	2	-.009	3	-.008	1	-2.748e-4	1	1452.023	2	7903.973	1
353		6	max	.01	3	.015	2	0	3	2.94e-5	3	NC	4	NC	2
354			min	-.01	2	-.014	3	-.008	1	-2.239e-4	1	1234.239	2	8500.411	1
355		7	max	.009	3	.02	2	0	3	3.279e-5	3	NC	4	NC	1
356			min	-.01	2	-.019	3	-.007	1	-1.729e-4	1	1100.753	2	NC	1
357		8	max	.009	3	.023	2	0	3	3.617e-5	3	NC	4	NC	1
358			min	-.01	2	-.022	3	-.006	1	-1.22e-4	1	1018.164	2	NC	1
359		9	max	.009	3	.025	2	0	3	3.956e-5	3	NC	4	NC	1
360			min	-.01	2	-.023	3	-.004	1	-7.103e-5	1	970.887	2	NC	1
361		10	max	.009	3	.026	2	0	3	4.295e-5	3	NC	4	NC	1
362			min	-.01	2	-.024	3	-.002	1	-2.007e-5	1	951.807	2	NC	1
363		11	max	.009	3	.026	2	.001	3	4.633e-5	3	NC	4	NC	1
364			min	-.01	2	-.023	3	0	1	1.144e-6	15	958.976	2	NC	1
365		12	max	.009	3	.024	2	.001	3	8.183e-5	1	NC	4	NC	1
366			min	-.01	2	-.021	3	0	15	3.562e-6	15	994.937	2	NC	1
367		13	max	.009	3	.021	2	.002	1	1.328e-4	1	NC	4	NC	1
368			min	-.01	2	-.018	3	0	15	5.979e-6	15	1068.123	2	NC	1
369		14	max	.009	3	.016	2	.003	1	1.837e-4	1	NC	4	NC	2
370			min	-.01	2	-.014	3	0	15	8.396e-6	15	1197.855	2	8639.183	1
371		15	max	.009	3	.01	2	.004	1	2.347e-4	1	NC	4	NC	2
372			min	-.01	2	-.008	3	0	15	1.081e-5	15	1429.516	2	8006.336	1
373		16	max	.009	3	.002	2	.004	1	2.692e-4	1	NC	4	NC	2
374			min	-.01	2	-.002	3	0	15	1.245e-5	15	1874.033	3	8286.803	1
375		17	max	.009	3	.006	3	.003	1	5.511e-5	3	NC	4	NC	2
376			min	-.01	2	-.008	2	0	15	-8.76e-5	1	2719.951	3	9973.358	1
377		18	max	.009	3	.014	3	.001	1	6.995e-3	2	NC	2	NC	1
378			min	-.01	2	-.019	2	0	15	-3.565e-3	3	5334.252	3	NC	1
379		19	max	.009	3	.023	3	0	3	1.412e-2	2	NC	1	NC	1
380			min	-.01	2	-.031	2	-.002	1	-7.254e-3	3	5718.141	2	NC	1
381	M5	1	max	.03	3	.087	3	.003	3	5.103e-6	3	NC	1	NC	1
382			min	-.033	2	-.075	2	-.003	1	0	2	3598.275	3	NC	1
383		2	max	.03	3	.053	3	.005	3	1.354e-4	3	NC	4	NC	1
384			min	-.033	2	-.045	2	-.003	1	-4.572e-5	1	1561.816	2	NC	1
385		3	max	.03	3	.021	3	.006	3	2.631e-4	3	NC	5	NC	1
386			min	-.033	2	-.017	2	-.003	1	-9.069e-5	1	800.842	2	NC	1
387		4	max	.03	3	.009	2	.007	3	2.527e-4	3	NC	5	NC	1
388			min	-.033	2	-.006	3	-.003	1	-8.652e-5	1	559.171	2	NC	1
389		5	max	.03	3	.031	2	.008	3	2.423e-4	3	NC	5	NC	1
390			min	-.033	2	-.028	3	-.003	1	-8.235e-5	1	442.325	2	9800.915	3
391		6	max	.03	3	.05	2	.008	3	2.319e-4	3	NC	5	NC	1
392			min	-.033	2	-.046	3	-.003	1	-7.818e-5	1	375.743	2	8827.321	3
393		7	max	.03	3	.065	2	.008	3	2.215e-4	3	NC	5	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.033	2	-.06	3	-.003	1	-7.402e-5	1	334.916	2	8367.71	3
395		8	max	.029	3	.076	2	.008	3	2.112e-4	3	NC	5	NC	1
396			min	-.033	2	-.069	3	-.002	1	-6.985e-5	1	309.636	2	8247.756	3
397		9	max	.029	3	.084	2	.008	3	2.008e-4	3	NC	5	NC	1
398			min	-.033	2	-.075	3	-.002	1	-6.568e-5	1	295.139	2	8400.139	3
399		10	max	.029	3	.087	2	.007	3	1.904e-4	3	NC	5	NC	1
400			min	-.033	2	-.077	3	-.002	1	-6.151e-5	1	289.247	2	8814.036	3
401		11	max	.029	3	.086	2	.007	3	1.8e-4	3	NC	5	NC	1
402			min	-.033	2	-.074	3	-.002	1	-5.734e-5	1	291.364	2	9521.898	3
403		12	max	.029	3	.08	2	.006	3	1.696e-4	3	NC	5	NC	1
404			min	-.033	2	-.068	3	-.002	1	-5.317e-5	1	302.264	2	NC	1
405		13	max	.029	3	.069	2	.005	3	1.592e-4	3	NC	5	NC	1
406			min	-.033	2	-.058	3	-.002	1	-4.9e-5	1	324.523	2	NC	1
407		14	max	.029	3	.054	2	.005	3	1.488e-4	3	NC	5	NC	1
408			min	-.033	2	-.044	3	-.002	1	-4.484e-5	1	364.057	2	NC	1
409		15	max	.029	3	.033	2	.004	3	1.384e-4	3	NC	5	NC	1
410			min	-.033	2	-.027	3	-.002	1	-4.067e-5	1	434.788	2	NC	1
411		16	max	.029	3	.006	2	.003	3	1.228e-4	3	NC	5	NC	1
412			min	-.033	2	-.006	3	-.002	1	-3.999e-5	1	575.994	2	NC	1
413		17	max	.029	3	.019	3	.002	3	-7.685e-8	10	NC	5	NC	1
414			min	-.033	2	-.026	2	-.002	1	-1.223e-4	1	855.501	3	NC	1
415		18	max	.029	3	.046	3	.001	3	-9.371e-8	10	NC	4	NC	1
416			min	-.033	2	-.063	2	-.001	1	-6.253e-5	1	1677.693	3	NC	1
417		19	max	.029	3	.074	3	0	3	-3.417e-8	15	NC	3	NC	1
418			min	-.033	2	-.103	2	-.001	1	-9.318e-7	3	1708.002	2	NC	1
419	M9	1	max	.01	3	.027	3	.003	3	1.383e-2	3	NC	1	NC	1
420			min	-.01	2	-.023	2	-.003	1	-9.429e-3	2	NC	1	NC	1
421		2	max	.01	3	.016	3	.001	3	6.804e-3	3	NC	4	NC	1
422			min	-.01	2	-.014	2	0	9	-4.628e-3	2	5120.472	2	NC	1
423		3	max	.01	3	.006	3	.002	1	1.798e-4	1	NC	4	NC	1
424			min	-.01	2	-.005	2	0	3	-9.007e-5	3	2626.827	2	NC	1
425		4	max	.01	3	.003	2	.003	1	1.367e-4	1	NC	4	NC	1
426			min	-.01	2	-.002	3	-.001	3	-9.217e-5	3	1835.471	2	NC	1
427		5	max	.01	3	.009	2	.003	1	9.361e-5	1	NC	4	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.009	3	.006	3	0	15	1.074e-4	3	NC	4	NC	2
452			min	-.01	2	-.008	2	-.006	1	-1.567e-4	1	2706.177	3	9642.487	1
453		18	max	.009	3	.015	3	0	15	3.648e-3	3	NC	2	NC	1
454			min	-.01	2	-.019	2	-.004	1	-7.001e-3	2	5307.77	3	NC	1
455		19	max	.009	3	.023	3	0	3	7.252e-3	3	NC	1	NC	1
456			min	-.01	2	-.031	2	-.001	1	-1.412e-2	2	5733.322	2	NC	1
457	M13	1	max	.003	1	.027	3	.01	3	3.982e-3	3	NC	1	NC	1
458			min	-.003	3	-.023	2	-.01	2	-3.477e-3	2	NC	1	NC	1
459		2	max	.003	1	.12	3	.01	9	4.968e-3	3	NC	4	NC	2
460			min	-.003	3	-.087	2	-.005	10	-4.357e-3	2	1347.235	3	8488.804	1
461		3	max	.003	1	.197	3	.029	1	5.954e-3	3	NC	5	NC	2
462			min	-.003	3	-.14	2	-.004	10	-5.238e-3	2	737.236	3	3734.267	1
463		4	max	.003	1	.248	3	.044	1	6.94e-3	3	NC	5	NC	3
464			min	-.003	3	-.175	2	-.003	10	-6.118e-3	2	569.555	3	2586.233	1
465		5	max	.003	1	.266	3	.05	1	7.926e-3	3	NC	5	NC	3
466			min	-.003	3	-.189	2	-.004	10	-6.998e-3	2	526.669	3	2300.171	1
467		6	max	.003	1	.252	3	.045	1	8.912e-3	3	NC	5	NC	2
468			min	-.003	3	-.181	2	-.007	10	-7.878e-3	2	558.988	3	2525.164	1
469		7	max	.003	1	.213	3	.03	1	9.898e-3	3	NC	5	NC	2
470			min	-.003	3	-.156	2	-.01	10	-8.758e-3	2	677.869	3	3596.525	1
471		8	max	.003	1	.159	3	.024	3	1.088e-2	3	NC	5	NC	2
472			min	-.003	3	-.122	2	-.019	2	-9.639e-3	2	951.05	3	8237.139	9
473		9	max	.003	1	.11	3	.027	3	1.187e-2	3	NC	4	NC	1
474			min	-.003	3	-.09	2	-.028	2	-1.052e-2	2	1522.428	3	6682.299	2
475		10	max	.003	1	.087	3	.03	3	1.286e-2	3	NC	4	NC	4
476			min	-.003	3	-.075	2	-.033	2	-1.14e-2	2	2102.47	3	5412.282	2
477		11	max	.003	1	.11	3	.033	3	1.187e-2	3	NC	4	NC	1
478			min	-.003	3	-.09	2	-.028	2	-1.052e-2	2	1522.426	3	5454.063	3
479		12	max	.003	1	.159	3	.034	3	1.089e-2	3	NC	5	NC	2
480			min	-.003	3	-.122	2	-.019	2	-9.639e-3	2	951.049	3	5177.655	3
481		13	max	.003	1	.213	3	.034	3	9.904e-3	3	NC	5	NC	2
482			min	-.003	3	-.156	2	-.01	10	-8.759e-3	2	677.868	3	3579.345	1
483		14	max	.003	1	.252	3	.045	1	8.92e-3	3	NC	5	NC	2
484			min	-.003	3	-.181	2	-.007	10	-7.879e-3	2	558.987	3	2522.671	1
485		15	max	.003	1	.266	3	.05	1	7.936e-3	3	NC	5	NC	5
486			min	-.003	3	-.189	2	-.004	10	-6.999e-3	2	526.668	3	2303.882	1
487		16	max	.003	1	.248	3	.044	1	6.952e-3	3	NC	5	NC	5
488			min	-.003	3	-.175	2	-.003	10	-6.119e-3	2	569.555	3	2597.064	1
489		17	max	.003	1	.198	3	.029	1	5.968e-3	3	NC	5	NC	2
490			min	-.003	3	-.14	2	-.004	10	-5.239e-3	2	737.235	3	3762.627	1
491		18	max	.003	1	.121	3	.013	3	4.984e-3	3	NC	4	NC	2
492			min	-.003	3	-.087	2	-.005	10	-4.359e-3	2	1347.234	3	8604.946	1
493		19	max	.003	1	.027	3	.01	3	4.e-3	3	NC	1	NC	1
494			min	-.003	3	-.023	2	-.01	2	-3.479e-3	2	NC	1	NC	1
495	M16	1	max	.001	1	.023	3	.009	3	4.528e-3	2	NC	1	NC	1
496			min	0	3	-.031	2	-.01	2	-3.363e-3	3	NC	1	NC	1
497		2	max	.001	1	.073	3	.013	3	5.683e-3	2	NC	4	NC	2
498			min	0	3	-.127	2	-.005	10	-4.171e-3	3	1318.405	2	8492.395	1
499		3	max	.001	1	.114	3	.029	1	6.837e-3	2	NC	5	NC	2
500			min	0	3	-.206	2	-.003	10	-4.979e-3	3	720.293	2	3735.225	1
501		4	max	.001	1	.142	3	.044	1	7.992e-3	2	NC	5	NC	5
502			min	0	3	-.258	2	-.003	10	-5.787e-3	3	554.852	2	2586.706	1
503		5	max	.001	1	.153	3	.05	1	9.146e-3	2	NC	5	NC	5
504			min	0	3	-.278	2	-.004	10	-6.596e-3	3	510.625	2	2300.542	1
505		6	max	.001	1	.149	3	.045	1	1.03e-2	2	NC	5	NC	2
506			min	0	3	-.265	2	-.007	10	-7.404e-3	3	537.67	2	2525.678	1
507		7	max	.001	1	.132	3	.031	3	1.146e-2	2	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-.227	2	-.01	10	-8.212e-3	3	642.986	2	3598.037	1
509	8	max	.001	1	.108	3	.031	3	1.261e-2	2	NC	5	NC	2
510		min	0	3	-.174	2	-.019	2	-9.02e-3	3	878.587	2	5749.648	3
511	9	max	.001	1	.085	3	.03	3	1.376e-2	2	NC	4	NC	1
512		min	0	3	-.125	2	-.028	2	-9.828e-3	3	1337.496	2	5986.39	3
513	10	max	.001	1	.074	3	.029	3	1.492e-2	2	NC	4	NC	4
514		min	0	3	-.103	2	-.033	2	-1.064e-2	3	1761.059	2	5458.373	2
515	11	max	.001	1	.085	3	.027	3	1.376e-2	2	NC	4	NC	1
516		min	0	3	-.125	2	-.028	2	-9.826e-3	3	1337.496	2	6750.477	2
517	12	max	.001	1	.108	3	.026	3	1.261e-2	2	NC	5	NC	2
518		min	0	3	-.174	2	-.019	2	-9.016e-3	3	878.587	2	7500.344	3
519	13	max	.001	1	.132	3	.03	1	1.146e-2	2	NC	5	NC	2
520		min	0	3	-.227	2	-.01	10	-8.206e-3	3	642.986	2	3592.176	1
521	14	max	.002	1	.149	3	.045	1	1.03e-2	2	NC	5	NC	2
522		min	0	3	-.265	2	-.007	10	-7.396e-3	3	537.67	2	2529.289	1
523	15	max	.002	1	.153	3	.05	1	9.148e-3	2	NC	5	NC	3
524		min	0	3	-.278	2	-.004	10	-6.586e-3	3	510.625	2	2309.224	1
525	16	max	.002	1	.141	3	.044	1	7.994e-3	2	NC	5	NC	3
526		min	0	3	-.258	2	-.003	10	-5.776e-3	3	554.852	2	2603.088	1
527	17	max	.002	1	.114	3	.029	1	6.84e-3	2	NC	5	NC	2
528		min	0	3	-.206	2	-.003	10	-4.966e-3	3	720.293	2	3772.435	1
529	18	max	.002	1	.072	3	.011	3	5.685e-3	2	NC	4	NC	2
530		min	0	3	-.127	2	-.005	10	-4.156e-3	3	1318.405	2	8634.397	1
531	19	max	.002	1	.023	3	.009	3	4.531e-3	2	NC	1	NC	1
532		min	0	3	-.031	2	-.01	2	-3.346e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	4.197e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.106e-5	2	NC	1	NC	1
535	2	max	0	3	-.002	15	.001	1	8.959e-4	3	NC	1	NC	1
536		min	0	2	-.007	4	0	3	-5.314e-4	2	NC	1	NC	1
537	3	max	0	3	-.003	15	.003	1	1.372e-3	3	NC	3	NC	1
538		min	0	2	-.013	4	-.004	3	-1.002e-3	2	5853.948	4	9634.994	3
539	4	max	0	3	-.004	15	.007	2	1.848e-3	3	NC	5	NC	4
540		min	0	2	-.019	4	-.008	3	-1.472e-3	2	4016.15	4	5364.485	3
541	5	max	0	3	-.006	15	.011	2	2.325e-3	3	NC	15	NC	4
542		min	0	2	-.024	4	-.013	3	-1.942e-3	2	3133.843	4	3541.991	3
543	6	max	0	3	-.007	15	.016	2	2.801e-3	3	NC	15	NC	4
544		min	-.001	2	-.029	4	-.019	3	-2.413e-3	2	2637.46	4	2589.723	3
545	7	max	0	3	-.008	15	.021	2	3.277e-3	3	9950.234	15	NC	4
546		min	-.001	2	-.033	4	-.025	3	-2.883e-3	2	2338.951	4	2030.621	3
547	8	max	0	3	-.008	15	.025	2	3.753e-3	3	9188.103	15	NC	4
548		min	-.002	2	-.036	4	-.031	3	-3.353e-3	2	2159.801	4	1678.166	3
549	9	max	0	3	-.009	15	.029	2	4.229e-3	3	8777.88	15	NC	4
550		min	-.002	2	-.037	4	-.036	3	-3.824e-3	2	2063.372	4	1447.112	3
551	10	max	0	3	-.009	15	.033	2	4.706e-3	3	8648.105	15	NC	4
552		min	-.002	2	-.038	4	-.04	3	-4.294e-3	2	2032.866	4	1294.506	3
553	11	max	.001	3	-.009	15	.035	2	5.182e-3	3	8777.88	15	NC	5
554		min	-.002	2	-.037	4	-.043	3	-4.764e-3	2	2063.372	4	1197.769	3
555	12	max	.001	3	-.008	15	.035	2	5.658e-3	3	9188.103	15	NC	5
556		min	-.002	2	-.036	4	-.043	3	-5.235e-3	2	2159.801	4	1145.729	3
557	13	max	.001	3	-.008	15	.033	2	6.134e-3	3	9950.234	15	NC	5
558		min	-.003	2	-.033	4	-.042	3	-5.705e-3	2	2338.951	4	1135.248	3
559	14	max	.001	3	-.007	15	.029	2	6.611e-3	3	NC	15	NC	4
560		min	-.003	2	-.029	4	-.038	3	-6.175e-3	2	2637.46	4	1171.418	3
561	15	max	.001	3	-.006	15	.024	1	7.087e-3	3	NC	15	NC	4
562		min	-.003	2	-.025	4	-.031	3	-6.646e-3	2	3133.843	4	1272.564	3
563	16	max	.002	3	-.005	15	.017	1	7.563e-3	3	NC	5	NC	4
564		min	-.003	2	-.02	4	-.02	3	-7.116e-3	2	4016.15	4	1488.279	3



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™
Software
Version 2.4.5673.0

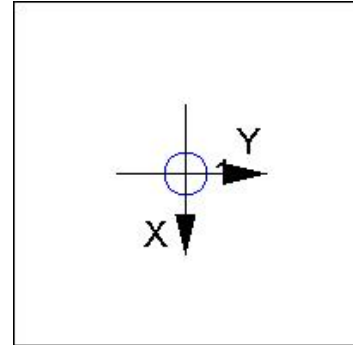
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Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f'_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-5)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16b)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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