

Schletter, Inc.	Standard PVMax Racking System Representative Calculations - ASCE 7-05	35° Tilt w/o Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 35°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	14.43 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.64	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.200	(Pressure)
$C_{f+ BOTTOM}$ =	2.000	
$C_{f- TOP, OUTER PURLIN}$ =	-2.700	
$C_{f- TOP, INNER PURLIN}$ =	-2.100	(Suction)
$C_{f- BOTTOM}$ =	-1.200	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	84 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-1.617 k-ft
$M_z$ =	0.000 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>58%</b>

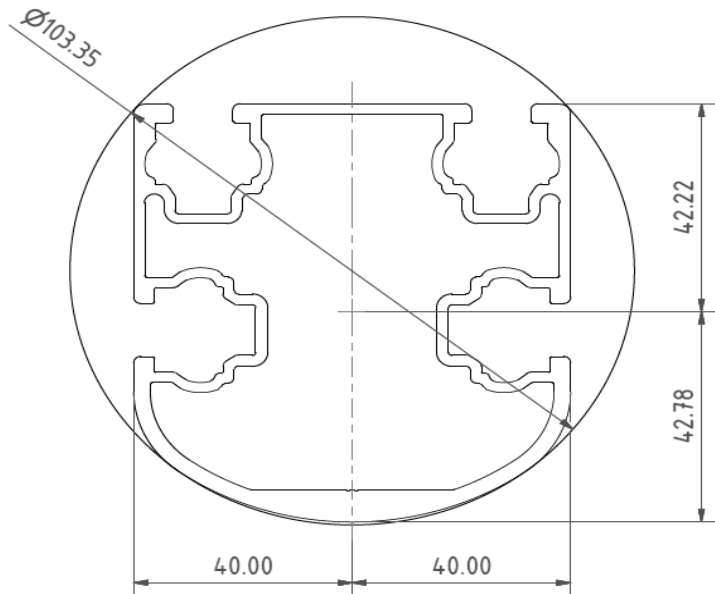


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-2.633 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.966 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>78%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.838 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>7%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.811 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>39%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	78.03 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.94 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.009 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.908 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	8.786 k
Utilization =	<b>34%</b>



### 5. FOUNDATION DESIGN CALCULATIONS

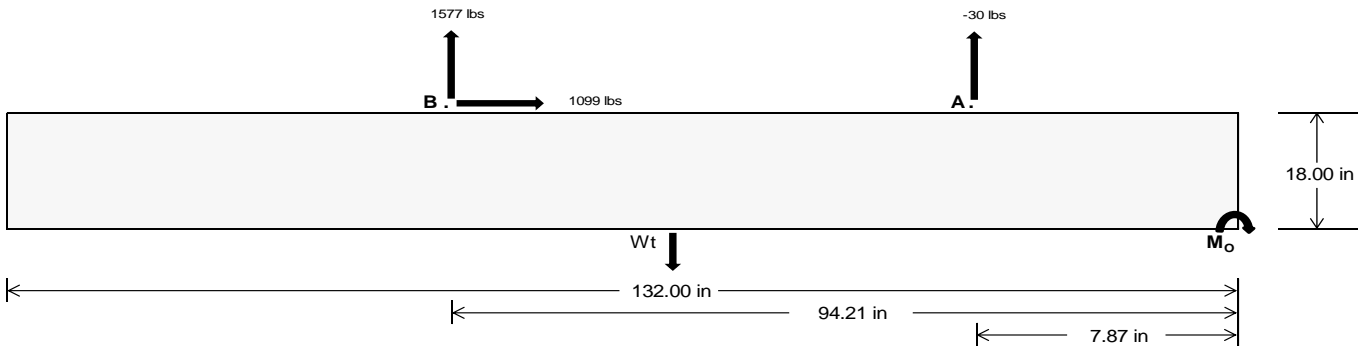
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =		<b>68.97</b>	<b>6565.48</b> k
Compressive Load =		<b>2389.05</b>	<b>4739.66</b> k
Lateral Load =		<b>7.28</b>	<b>4573.21</b> k
Moment (Weak Axis) =		<b>0.01</b>	<b>0.00</b> k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 168101.3$  in-lbs  
Resisting Force Required = 2546.99 lbs  
S.F. = 1.67  
Weight Required = 4244.98 lbs  
Minimum Width = 33 in  
Weight Provided = 6579.38 lbs

### Sliding

Force = 1099.37 lbs  
Friction = 0.4  
Weight Required = 2748.44 lbs  
Resisting Weight = 6579.38 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 1099.37 lbs  
Cohesion = 130 psf  
Area = 30.25 ft<sup>2</sup>  
Resisting = 3289.69 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (2) #5 rebar.

A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 33in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 33in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
33 in 34 in 35 in 36 in  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(2.75 \text{ ft}) =$  6579 lbs 6779 lbs 6978 lbs 7178 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in	33 in	34 in	35 in	36 in
$F_A$	760 lbs	760 lbs	760 lbs	760 lbs	1019 lbs	1019 lbs	1019 lbs	1019 lbs	1238 lbs	1238 lbs	1238 lbs	1238 lbs	61 lbs	61 lbs	61 lbs	61 lbs
$F_B$	667 lbs	667 lbs	667 lbs	667 lbs	2164 lbs	2164 lbs	2164 lbs	2164 lbs	2039 lbs	2039 lbs	2039 lbs	2039 lbs	-3154 lbs	-3154 lbs	-3154 lbs	-3154 lbs
$F_V$	106 lbs	106 lbs	106 lbs	106 lbs	1985 lbs	1985 lbs	1985 lbs	1985 lbs	1556 lbs	1556 lbs	1556 lbs	1556 lbs	-2199 lbs	-2199 lbs	-2199 lbs	-2199 lbs
$P_{total}$	8006 lbs	8206 lbs	8405 lbs	8605 lbs	9763 lbs	9962 lbs	10162 lbs	10361 lbs	9857 lbs	10056 lbs	10256 lbs	10455 lbs	855 lbs	974 lbs	1094 lbs	1214 lbs
$M$	2270 lbs-ft	2270 lbs-ft	2270 lbs-ft	2270 lbs-ft	2826 lbs-ft	2826 lbs-ft	2826 lbs-ft	2826 lbs-ft	3538 lbs-ft	3538 lbs-ft	3538 lbs-ft	3538 lbs-ft	4410 lbs-ft	4410 lbs-ft	4410 lbs-ft	4410 lbs-ft
$e$	0.28 ft	0.28 ft	0.27 ft	0.26 ft	0.29 ft	0.28 ft	0.28 ft	0.27 ft	0.36 ft	0.35 ft	0.34 ft	0.34 ft	5.16 ft	4.53 ft	4.03 ft	3.63 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	223.7 psf	223.6 psf	223.4 psf	223.2 psf	271.8 psf	270.2 psf	268.7 psf	267.3 psf	262.1 psf	260.7 psf	259.5 psf	258.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	305.6 psf	303.0 psf	300.6 psf	298.3 psf	373.7 psf	369.1 psf	364.8 psf	360.7 psf	389.6 psf	384.6 psf	379.8 psf	375.3 psf	608.2 psf	235.4 psf	170.3 psf	144.5 psf

Maximum Bearing Pressure = 608 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

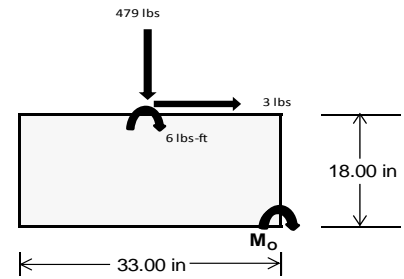
## Overturning Check

$M_o = 647.5 \text{ ft-lbs}$   
 Resisting Force Required = 470.90 lbs  
 S.F. = 1.67  
 Weight Required = 784.84 lbs  
 Minimum Width = 33 in  
 Weight Provided = 6579.38 lbs

*A minimum 132in long x 33in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	33 in			33 in			33 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	196 lbs	446 lbs	196 lbs	479 lbs	1225 lbs	479 lbs	57 lbs	131 lbs	57 lbs
$F_v$	1 lbs	0 lbs	1 lbs	3 lbs	0 lbs	3 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	8341 lbs	6579 lbs	8341 lbs	8232 lbs	6579 lbs	8232 lbs	2439 lbs	6579 lbs	2439 lbs
$M$	4 lbs-ft	0 lbs-ft	4 lbs-ft	11 lbs-ft	0 lbs-ft	11 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft	0.46 ft
$f_{min}$	275.5 psf	217.5 psf	275.5 psf	271.4 psf	217.5 psf	271.4 psf	80.6 psf	217.5 psf	80.6 psf
$f_{max}$	276.0 psf	217.5 psf	276.0 psf	272.9 psf	217.5 psf	272.9 psf	80.7 psf	217.5 psf	80.7 psf



Maximum Bearing Pressure = 276 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 33in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 33in wide x 18in tall ballast foundation and fiber reinforcing with (2) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.923 k
Allowable Uplift =	1.214 k
Utilization =	<u>76%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.320 k
Allowable Uplift =	4.357 k
Utilization =	<u>53%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.838 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>25%</u>

#### Rear Strut

Maximum Axial Load =	4.337 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>58%</u>

#### Diagonal Strut

Maximum Axial Load =	2.869 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>39%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	53.78 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	1.076 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 84 \text{ in}$$

$$J = 0.432$$

$$232.383$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 28.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 84$$

$$J = 0.432$$

$$147.782$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = 0.942$$

$$38.7028$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.57371$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.87952$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 28.0279 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_c[Bp - 1.6Dp^*b/t]$$

$$\phi_{FL} = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y Fcy$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 28.03 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 28.85 \text{ kips}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

$$\text{Strut} = \underline{\underline{55 \times 55}}$$

### Strong Axis:

#### 3.4.14

$$L_b = 86.60 \text{ in}$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6 \text{ ksi}$$

### Weak Axis:

#### 3.4.14

$$L_b = 86.6$$

$$J = 0.942$$

$$135.148$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{Cc}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi_{FL} = \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}]$$

$$\phi_{FL} = 29.6$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{max} St = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp * b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \text{ in} \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 78.03 \\ J &= 0.942 \\ &= 121.773 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.8 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.80509$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83271$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 8.94465 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi_y Fcy$$

$$\phi F_L = 33.25 \text{ ksi}$$

$$\phi F_L = 8.94 \text{ ksi}$$

$$A = 663.99 \text{ mm}^2$$

$$1.03 \text{ in}^2$$

$$P_{\max} = 9.21 \text{ kips}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-32.97	-32.97	0	0
2	M14	Y	-32.97	-32.97	0	0
3	M15	Y	-32.97	-32.97	0	0
4	M16	Y	-32.97	-32.97	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-75.661	-75.661	0	0
2	M14	y	-75.661	-75.661	0	0
3	M15	y	-126.102	-126.102	0	0
4	M16	y	-126.102	-126.102	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	170.238	170.238	0	0
2	M14	y	132.407	132.407	0	0
3	M15	y	75.661	75.661	0	0
4	M16	y	75.661	75.661	0	0

### Load Combinations

	Description	S...	P...	S...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...	B...	Fa...
1	LRFD 1.2D + 1.6S + 0.8W	Yes	Y		1	1.2	3	1.6	4	.8										
2	LRFD 1.2D + 1.6W + 0.5S	Yes	Y		1	1.2	3	.5	4	1.6										
3	LRFD 0.9D + 1.6W	Yes	Y		2	.9					5	1.6								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes	Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes	Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes	Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes	Y		1	.56					6	1.25								



RISA-3D Version 13.0.0 [T:\...\PVMax 60 Cell 2V 35° 120mph 30psf 7ft 7-05 NS.r3d] Page 19



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	49.177	1	191.461	2	1.993	3	.012	2	-.004	15	.723	3
28			min	2.828	15	-335.165	3	-22.191	1	0	15	-.077	1	-.346	2
29		15	max	49.177	1	79.256	2	6.626	1	.012	2	-.004	12	.903	3
30			min	2.828	15	-126.727	3	-.552	10	0	15	-.083	1	-.452	2
31		16	max	49.177	1	81.71	3	35.443	1	.012	2	-.001	12	.92	3
32			min	2.828	15	-32.949	2	2.04	15	0	15	-.067	1	-.47	2
33		17	max	49.177	1	290.148	3	64.261	1	.012	2	.004	3	.776	3
34			min	2.828	15	-145.154	2	3.634	15	0	15	-.028	1	-.4	2
35		18	max	49.177	1	498.585	3	93.078	1	.012	2	.033	1	.469	3
36			min	2.828	15	-257.359	2	5.227	15	0	15	.001	10	-.244	2
37		19	max	49.177	1	707.023	3	121.895	1	.012	2	.117	1	0	2
38			min	2.828	15	-369.564	2	6.821	15	0	15	.007	15	0	3
39	M14	1	max	28.677	1	428.997	2	-7.102	15	.011	3	.14	1	0	2
40			min	1.636	15	-586.55	3	-126.899	1	-.012	2	.008	15	0	3
41		2	max	28.677	1	316.792	2	-5.508	15	.011	3	.052	1	.393	3
42			min	1.636	15	-425.191	3	-98.082	1	-.012	2	.003	15	-.29	2
43		3	max	28.677	1	204.587	2	-3.914	15	.011	3	.006	3	.661	3
44			min	1.636	15	-263.832	3	-69.265	1	-.012	2	-.013	1	-.493	2
45		4	max	28.677	1	92.382	2	-2.32	15	.011	3	0	3	.804	3
46			min	1.636	15	-102.473	3	-40.447	1	-.012	2	-.055	1	-.608	2
47		5	max	28.677	1	58.886	3	-.133	10	.011	3	-.003	12	.821	3
48			min	1.636	15	-19.823	2	-11.63	1	-.012	2	-.076	1	-.636	2
49		6	max	28.677	1	220.245	3	17.187	1	.011	3	-.004	15	.712	3
50			min	1.636	15	-132.028	2	-2.473	3	-.012	2	-.073	1	-.577	2
51		7	max	28.677	1	381.604	3	46.005	1	.011	3	-.003	15	.478	3
52			min	1.636	15	-244.233	2	-.082	3	-.012	2	-.049	1	-.431	2
53		8	max	28.677	1	542.963	3	74.822	1	.011	3	.004	2	.119	3
54			min	1.636	15	-356.437	2	1.693	12	-.012	2	-.008	3	-.198	2
55		9	max	28.677	1	704.322	3	103.639	1	.011	3	.068	1	.123	2
56			min	1.636	15	-468.642	2	3.287	12	-.012	2	-.005	3	-.366	3
57		10	max	28.677	1	580.847	2	-4.881	12	.011	3	.159	1	.531	2
58			min	1.636	15	-865.681	3	-132.457	1	-.012	2	0	3	-.977	3
59		11	max	28.677	1	468.642	2	-3.287	12	.012	2	.068	1	.123	2
60			min	1.636	15	-704.322	3	-103.639	1	-.011	3	-.005	3	-.366	3
61		12	max	28.677	1	356.437	2	-1.693	12	.012	2	.004	2	.119	3
62			min	1.636	15	-542.963	3	-74.822	1	-.011	3	-.008	3	-.198	2
63		13	max	28.677	1	244.233	2	.082	3	.012	2	-.003	15	.478	3
64			min	1.636	15	-381.604	3	-46.005	1	-.011	3	-.049	1	-.431	2
65		14	max	28.677	1	132.028	2	2.473	3	.012	2	-.004	15	.712	3
66			min	1.636	15	-220.245	3	-17.187	1	-.011	3	-.073	1	-.577	2
67		15	max	28.677	1	19.823	2	11.63	1	.012	2	-.003	12	.821	3
68			min	1.636	15	-58.886	3	.133	10	-.011	3	-.076	1	-.636	2
69		16	max	28.677	1	102.473	3	40.447	1	.012	2	0	3	.804	3
70			min	1.636	15	-92.382	2	2.32	15	-.011	3	-.055	1	-.608	2
71		17	max	28.677	1	263.832	3	69.265	1	.012	2	.006	3	.661	3
72			min	1.636	15	-204.587	2	3.914	15	-.011	3	-.013	1	-.493	2
73		18	max	28.677	1	425.191	3	98.082	1	.012	2	.052	1	.393	3
74			min	1.636	15	-316.792	2	5.508	15	-.011	3	.003	15	-.29	2
75		19	max	28.677	1	586.55	3	126.899	1	.012	2	.14	1	0	2
76			min	1.636	15	-428.997	2	7.102	15	-.011	3	.008	15	0	3
77	M15	1	max	-1.7	15	640.919	2	-7.1	15	.012	2	.14	1	0	2
78			min	-29.467	1	-348.553	3	-126.94	1	-.01	3	.008	15	0	3
79		2	max	-1.7	15	465.943	2	-5.506	15	.012	2	.053	1	.236	3
80			min	-29.467	1	-257.811	3	-98.123	1	-.01	3	.003	15	-.43	2
81		3	max	-1.7	15	290.967	2	-3.912	15	.012	2	.006	3	.401	3
82			min	-29.467	1	-167.069	3	-69.305	1	-.01	3	-.013	1	-.725	2
83		4	max	-1.7	15	115.991	2	-2.318	15	.012	2	0	12	.496	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-29.467	1	-76.327	3	-40.488	1	-.01	3	-.055	1	-.883	2
85		5	max	-1.7	15	14.414	3	-.233	10	.012	2	-.003	12	.52	3
86			min	-29.467	1	-58.985	2	-11.671	1	-.01	3	-.076	1	-.905	2
87		6	max	-1.7	15	105.156	3	17.147	1	.012	2	-.004	15	.473	3
88			min	-29.467	1	-233.961	2	-2.159	3	-.01	3	-.073	1	-.791	2
89		7	max	-1.7	15	195.898	3	45.964	1	.012	2	-.003	15	.356	3
90			min	-29.467	1	-408.937	2	.231	3	-.01	3	-.049	1	-.541	2
91		8	max	-1.7	15	286.64	3	74.781	1	.012	2	.004	2	.169	3
92			min	-29.467	1	-583.913	2	1.89	12	-.01	3	-.007	3	-.155	2
93		9	max	-1.7	15	377.381	3	103.599	1	.012	2	.067	1	.367	2
94			min	-29.467	1	-758.889	2	3.483	12	-.01	3	-.004	3	-.09	3
95		10	max	-1.7	15	933.865	2	-5.077	12	.012	2	.159	1	1.025	2
96			min	-29.467	1	-468.123	3	-132.416	1	-.01	3	0	3	-.418	3
97		11	max	-1.7	15	758.889	2	-3.483	12	.01	3	.067	1	.367	2
98			min	-29.467	1	-377.381	3	-103.599	1	-.012	2	-.004	3	-.09	3
99		12	max	-1.7	15	583.913	2	-1.89	12	.01	3	.004	2	.169	3
100			min	-29.467	1	-286.64	3	-74.781	1	-.012	2	-.007	3	-.155	2
101		13	max	-1.7	15	408.937	2	-.231	3	.01	3	-.003	15	.356	3
102			min	-29.467	1	-195.898	3	-45.964	1	-.012	2	-.049	1	-.541	2
103		14	max	-1.7	15	233.961	2	2.159	3	.01	3	-.004	15	.473	3
104			min	-29.467	1	-105.156	3	-17.147	1	-.012	2	-.073	1	-.791	2
105		15	max	-1.7	15	58.985	2	11.671	1	.01	3	-.003	12	.52	3
106			min	-29.467	1	-14.414	3	.233	10	-.012	2	-.076	1	-.905	2
107		16	max	-1.7	15	76.327	3	40.488	1	.01	3	0	12	.496	3
108			min	-29.467	1	-115.991	2	2.318	15	-.012	2	-.055	1	-.883	2
109		17	max	-1.7	15	167.069	3	69.305	1	.01	3	.006	3	.401	3
110			min	-29.467	1	-290.967	2	3.912	15	-.012	2	-.013	1	-.725	2
111		18	max	-1.7	15	257.811	3	98.123	1	.01	3	.053	1	.236	3
112			min	-29.467	1	-465.943	2	5.506	15	-.012	2	.003	15	-.43	2
113		19	max	-1.7	15	348.553	3	126.94	1	.01	3	.14	1	0	2
114			min	-29.467	1	-640.919	2	7.1	15	-.012	2	.008	15	0	3
115	M16	1	max	-3.071	15	584.473	2	-6.829	15	.007	2	.118	1	0	2
116			min	-53.678	1	-296.753	3	-122.322	1	-.011	3	.007	15	0	3
117		2	max	-3.071	15	409.497	2	-5.235	15	.007	2	.034	1	.196	3
118			min	-53.678	1	-206.011	3	-93.505	1	-.011	3	.002	15	-.387	2
119		3	max	-3.071	15	234.521	2	-3.641	15	.007	2	.003	3	.32	3
120			min	-53.678	1	-115.269	3	-64.688	1	-.011	3	-.027	1	-.637	2
121		4	max	-3.071	15	59.545	2	-2.047	15	.007	2	-.002	12	.375	3
122			min	-53.678	1	-24.528	3	-35.87	1	-.011	3	-.066	1	-.751	2
123		5	max	-3.071	15	66.214	3	.175	10	.007	2	-.004	12	.359	3
124			min	-53.678	1	-115.431	2	-7.053	1	-.011	3	-.083	1	-.73	2
125		6	max	-3.071	15	156.956	3	21.764	1	.007	2	-.004	15	.272	3
126			min	-53.678	1	-290.407	2	-.958	3	-.011	3	-.077	1	-.572	2
127		7	max	-3.071	15	247.698	3	50.582	1	.007	2	-.003	15	.114	3
128			min	-53.678	1	-465.383	2	1.064	12	-.011	3	-.049	1	-.278	2
129		8	max	-3.071	15	338.439	3	79.399	1	.007	2	.005	2	.152	2
130			min	-53.678	1	-640.359	2	2.658	12	-.011	3	-.006	3	-.113	3
131		9	max	-3.071	15	429.181	3	108.216	1	.007	2	.075	1	.718	2
132			min	-53.678	1	-815.335	2	4.251	12	-.011	3	-.002	3	-.412	3
133		10	max	-3.071	15	990.311	2	-5.845	12	.007	2	.17	1	1.42	2
134			min	-53.678	1	-519.923	3	-137.034	1	-.011	3	.003	12	-.781	3
135		11	max	-3.071	15	815.335	2	-4.251	12	.011	3	.075	1	.718	2
136			min	-53.678	1	-429.181	3	-108.216	1	-.007	2	-.002	3	-.412	3
137		12	max	-3.071	15	640.359	2	-2.658	12	.011	3	.005	2	.152	2
138			min	-53.678	1	-338.439	3	-79.399	1	-.007	2	-.006	3	-.113	3
139		13	max	-3.071	15	465.383	2	-1.064	12	.011	3	-.003	15	.114	3
140			min	-53.678	1	-247.698	3	-50.582	1	-.007	2	-.049	1	-.278	2



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-3.071	15	290.407	2	.958	3	.011	3	-.004	15	.272	3
142			min	-53.678	1	-156.956	3	-21.764	1	-.007	2	-.077	1	-.572	2
143		15	max	-3.071	15	115.431	2	7.053	1	.011	3	-.004	12	.359	3
144			min	-53.678	1	-66.214	3	-.175	10	-.007	2	-.083	1	-.73	2
145		16	max	-3.071	15	24.528	3	35.87	1	.011	3	-.002	12	.375	3
146			min	-53.678	1	-59.545	2	2.047	15	-.007	2	-.066	1	-.751	2
147		17	max	-3.071	15	115.269	3	64.688	1	.011	3	.003	3	.32	3
148			min	-53.678	1	-234.521	2	3.641	15	-.007	2	-.027	1	-.637	2
149		18	max	-3.071	15	206.011	3	93.505	1	.011	3	.034	1	.196	3
150			min	-53.678	1	-409.497	2	5.235	15	-.007	2	.002	15	-.387	2
151		19	max	-3.071	15	296.753	3	122.322	1	.011	3	.118	1	0	2
152			min	-53.678	1	-584.473	2	6.829	15	-.007	2	.007	15	0	3
153	M2	1	max	947.831	2	2.02	4	.165	1	0	3	0	3	0	1
154			min	-1382.186	3	.475	15	.009	15	0	1	0	2	0	1
155		2	max	948.352	2	1.901	4	.165	1	0	3	0	1	0	15
156			min	-1381.795	3	.447	15	.009	15	0	1	0	10	0	4
157		3	max	948.873	2	1.782	4	.165	1	0	3	0	1	0	15
158			min	-1381.405	3	.419	15	.009	15	0	1	0	15	-.001	4
159		4	max	949.393	2	1.663	4	.165	1	0	3	0	1	0	15
160			min	-1381.014	3	.391	15	.009	15	0	1	0	15	-.002	4
161		5	max	949.914	2	1.544	4	.165	1	0	3	0	1	0	15
162			min	-1380.624	3	.363	15	.009	15	0	1	0	15	-.003	4
163		6	max	950.435	2	1.425	4	.165	1	0	3	0	1	0	15
164			min	-1380.233	3	.335	15	.009	15	0	1	0	15	-.003	4
165		7	max	950.956	2	1.306	4	.165	1	0	3	0	1	0	15
166			min	-1379.843	3	.307	15	.009	15	0	1	0	15	-.004	4
167		8	max	951.476	2	1.187	4	.165	1	0	3	0	1	0	15
168			min	-1379.452	3	.279	15	.009	15	0	1	0	15	-.004	4
169		9	max	951.997	2	1.069	4	.165	1	0	3	0	1	-.001	15
170			min	-1379.062	3	.242	12	.009	15	0	1	0	15	-.004	4
171		10	max	952.518	2	.95	4	.165	1	0	3	0	1	-.001	15
172			min	-1378.671	3	.196	12	.009	15	0	1	0	15	-.005	4
173		11	max	953.038	2	.852	2	.165	1	0	3	0	1	-.001	15
174			min	-1378.28	3	.15	12	.009	15	0	1	0	15	-.005	4
175		12	max	953.559	2	.759	2	.165	1	0	3	0	1	-.001	15
176			min	-1377.89	3	.104	12	.009	15	0	1	0	15	-.005	4
177		13	max	954.08	2	.666	2	.165	1	0	3	0	1	-.001	15
178			min	-1377.499	3	.057	12	.009	15	0	1	0	15	-.006	4
179		14	max	954.6	2	.574	2	.165	1	0	3	0	1	-.001	15
180			min	-1377.109	3	-.001	3	.009	15	0	1	0	15	-.006	4
181		15	max	955.121	2	.481	2	.165	1	0	3	0	1	-.001	15
182			min	-1376.718	3	-.071	3	.009	15	0	1	0	15	-.006	4
183		16	max	955.642	2	.389	2	.165	1	0	3	0	1	-.001	15
184			min	-1376.328	3	-.14	3	.009	15	0	1	0	15	-.006	4
185		17	max	956.163	2	.296	2	.165	1	0	3	0	1	-.001	12
186			min	-1375.937	3	-.21	3	.009	15	0	1	0	15	-.006	4
187		18	max	956.683	2	.203	2	.165	1	0	3	0	1	-.001	12
188			min	-1375.547	3	-.279	3	.009	15	0	1	0	15	-.006	4
189		19	max	957.204	2	.111	2	.165	1	0	3	.001	1	-.001	12
190			min	-1375.156	3	-.348	3	.009	15	0	1	0	15	-.006	4
191	M3	1	max	856.557	2	7.663	4	.169	1	0	3	0	1	.006	4
192			min	-951.878	3	1.801	15	.01	15	0	1	0	15	.001	12
193		2	max	856.386	2	6.902	4	.169	1	0	3	0	1	.004	2
194			min	-952.006	3	1.623	15	.01	15	0	1	0	15	0	3
195		3	max	856.216	2	6.141	4	.169	1	0	3	0	1	.001	2
196			min	-952.134	3	1.444	15	.01	15	0	1	0	15	-.001	3
197		4	max	856.046	2	5.38	4	.169	1	0	3	0	1	0	15





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198			min	-952.262	3	1.265	15	.01	15	0	1	0	15	-.003	3
199		5	max	855.875	2	4.619	4	.169	1	0	3	0	1	0	15
200			min	-952.389	3	1.086	15	.01	15	0	1	0	15	-.004	4
201		6	max	855.705	2	3.858	4	.169	1	0	3	0	1	-.001	15
202			min	-952.517	3	.907	15	.01	15	0	1	0	15	-.006	4
203		7	max	855.535	2	3.097	4	.169	1	0	3	0	1	-.002	15
204			min	-952.645	3	.728	15	.01	15	0	1	0	15	-.007	4
205		8	max	855.364	2	2.336	4	.169	1	0	3	0	1	-.002	15
206			min	-952.773	3	.549	15	.01	15	0	1	0	15	-.008	4
207		9	max	855.194	2	1.575	4	.169	1	0	3	0	1	-.002	15
208			min	-952.9	3	.37	15	.01	15	0	1	0	15	-.009	4
209		10	max	855.024	2	.814	4	.169	1	0	3	0	1	-.002	15
210			min	-953.028	3	.168	12	.01	15	0	1	0	15	-.01	4
211		11	max	854.853	2	.216	2	.169	1	0	3	0	1	-.002	15
212			min	-953.156	3	-.208	3	.01	15	0	1	0	15	-.01	4
213		12	max	854.683	2	-.166	15	.169	1	0	3	0	1	-.002	15
214			min	-953.284	3	-.708	4	.01	15	0	1	0	15	-.01	4
215		13	max	854.513	2	-.345	15	.169	1	0	3	.001	1	-.002	15
216			min	-953.412	3	-1.469	4	.01	15	0	1	0	15	-.009	4
217		14	max	854.342	2	-.524	15	.169	1	0	3	.001	1	-.002	15
218			min	-953.539	3	-2.23	4	.01	15	0	1	0	15	-.009	4
219		15	max	854.172	2	-.703	15	.169	1	0	3	.001	1	-.002	15
220			min	-953.667	3	-2.991	4	.01	15	0	1	0	15	-.008	4
221		16	max	854.002	2	-.882	15	.169	1	0	3	.001	1	-.001	15
222			min	-953.795	3	-3.752	4	.01	15	0	1	0	15	-.006	4
223		17	max	853.831	2	-1.061	15	.169	1	0	3	.001	1	-.001	15
224			min	-953.923	3	-4.513	4	.01	15	0	1	0	15	-.004	4
225		18	max	853.661	2	-1.239	15	.169	1	0	3	.001	1	0	15
226			min	-954.05	3	-5.274	4	.01	15	0	1	0	15	-.002	4
227		19	max	853.491	2	-1.418	15	.169	1	0	3	.001	1	0	1
228			min	-954.178	3	-6.035	4	.01	15	0	1	0	15	0	1
229	M4	1	max	711.046	1	0	1	-.325	15	0	1	.001	1	0	1
230			min	33.373	15	0	1	-5.707	1	0	1	0	15	0	1
231		2	max	711.216	1	0	1	-.325	15	0	1	0	1	0	1
232			min	33.425	15	0	1	-5.707	1	0	1	0	15	0	1
233		3	max	711.387	1	0	1	-.325	15	0	1	0	1	0	1
234			min	33.476	15	0	1	-5.707	1	0	1	0	10	0	1
235		4	max	711.557	1	0	1	-.325	15	0	1	0	15	0	1
236			min	33.527	15	0	1	-5.707	1	0	1	0	1	0	1
237		5	max	711.727	1	0	1	-.325	15	0	1	0	15	0	1
238			min	33.579	15	0	1	-5.707	1	0	1	-.001	1	0	1
239		6	max	711.898	1	0	1	-.325	15	0	1	0	15	0	1
240			min	33.63	15	0	1	-5.707	1	0	1	-.002	1	0	1
241		7	max	712.068	1	0	1	-.325	15	0	1	0	15	0	1
242			min	33.682	15	0	1	-5.707	1	0	1	-.003	1	0	1
243		8	max	712.238	1	0	1	-.325	15	0	1	0	15	0	1
244			min	33.733	15	0	1	-5.707	1	0	1	-.003	1	0	1
245		9	max	712.409	1	0	1	-.325	15	0	1	0	15	0	1
246			min	33.784	15	0	1	-5.707	1	0	1	-.004	1	0	1
247		10	max	712.579	1	0	1	-.325	15	0	1	0	15	0	1
248			min	33.836	15	0	1	-5.707	1	0	1	-.005	1	0	1
249		11	max	712.749	1	0	1	-.325	15	0	1	0	15	0	1
250			min	33.887	15	0	1	-5.707	1	0	1	-.005	1	0	1
251		12	max	712.92	1	0	1	-.325	15	0	1	0	15	0	1
252			min	33.939	15	0	1	-5.707	1	0	1	-.006	1	0	1
253		13	max	713.09	1	0	1	-.325	15	0	1	0	15	0	1
254			min	33.99	15	0	1	-5.707	1	0	1	-.006	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	713.261	1	0	1	-.325	15	0	1	0	15	0	1
256		min	34.041	15	0	1	-5.707	1	0	1	-.007	1	0	1
257	15	max	713.431	1	0	1	-.325	15	0	1	0	15	0	1
258		min	34.093	15	0	1	-5.707	1	0	1	-.008	1	0	1
259	16	max	713.601	1	0	1	-.325	15	0	1	0	15	0	1
260		min	34.144	15	0	1	-5.707	1	0	1	-.008	1	0	1
261	17	max	713.772	1	0	1	-.325	15	0	1	0	15	0	1
262		min	34.196	15	0	1	-5.707	1	0	1	-.009	1	0	1
263	18	max	713.942	1	0	1	-.325	15	0	1	0	15	0	1
264		min	34.247	15	0	1	-5.707	1	0	1	-.01	1	0	1
265	19	max	714.112	1	0	1	-.325	15	0	1	0	15	0	1
266		min	34.298	15	0	1	-5.707	1	0	1	-.01	1	0	1
267	M6	1	max	2898.884	2	2.228	2	0	1	0	0	1	0	1
268		min	-4337.392	3	.253	12	0	1	0	1	0	1	0	1
269	2	max	2899.405	2	2.135	2	0	1	0	1	0	1	0	12
270		min	-4337.002	3	.207	12	0	1	0	1	0	1	0	2
271	3	max	2899.926	2	2.042	2	0	1	0	1	0	1	0	12
272		min	-4336.611	3	.161	12	0	1	0	1	0	1	-.002	2
273	4	max	2900.446	2	1.95	2	0	1	0	1	0	1	0	12
274		min	-4336.221	3	.114	12	0	1	0	1	0	1	-.002	2
275	5	max	2900.967	2	1.857	2	0	1	0	1	0	1	0	12
276		min	-4335.83	3	.049	3	0	1	0	1	0	1	-.003	2
277	6	max	2901.488	2	1.765	2	0	1	0	1	0	1	0	12
278		min	-4335.44	3	-.021	3	0	1	0	1	0	1	-.004	2
279	7	max	2902.008	2	1.672	2	0	1	0	1	0	1	0	12
280		min	-4335.049	3	-.09	3	0	1	0	1	0	1	-.004	2
281	8	max	2902.529	2	1.579	2	0	1	0	1	0	1	0	3
282		min	-4334.659	3	-.16	3	0	1	0	1	0	1	-.005	2
283	9	max	2903.05	2	1.487	2	0	1	0	1	0	1	0	3
284		min	-4334.268	3	-.229	3	0	1	0	1	0	1	-.005	2
285	10	max	2903.571	2	1.394	2	0	1	0	1	0	1	0	3
286		min	-4333.878	3	-.299	3	0	1	0	1	0	1	-.006	2
287	11	max	2904.091	2	1.302	2	0	1	0	1	0	1	0	3
288		min	-4333.487	3	-.368	3	0	1	0	1	0	1	-.006	2
289	12	max	2904.612	2	1.209	2	0	1	0	1	0	1	0	3
290		min	-4333.097	3	-.438	3	0	1	0	1	0	1	-.007	2
291	13	max	2905.133	2	1.116	2	0	1	0	1	0	1	0	3
292		min	-4332.706	3	-.507	3	0	1	0	1	0	1	-.007	2
293	14	max	2905.653	2	1.024	2	0	1	0	1	0	1	0	3
294		min	-4332.316	3	-.576	3	0	1	0	1	0	1	-.008	2
295	15	max	2906.174	2	.931	2	0	1	0	1	0	1	0	3
296		min	-4331.925	3	-.646	3	0	1	0	1	0	1	-.008	2
297	16	max	2906.695	2	.838	2	0	1	0	1	0	1	.001	3
298		min	-4331.535	3	-.715	3	0	1	0	1	0	1	-.008	2
299	17	max	2907.215	2	.746	2	0	1	0	1	0	1	.001	3
300		min	-4331.144	3	-.785	3	0	1	0	1	0	1	-.008	2
301	18	max	2907.736	2	.653	2	0	1	0	1	0	1	.002	3
302		min	-4330.754	3	-.854	3	0	1	0	1	0	1	-.009	2
303	19	max	2908.257	2	.561	2	0	1	0	1	0	1	.002	3
304		min	-4330.363	3	-.924	3	0	1	0	1	0	1	-.009	2
305	M7	1	max	2810.836	2	7.681	4	0	1	0	0	1	.009	2
306		min	-2866.24	3	1.804	15	0	1	0	1	0	1	-.002	3
307	2	max	2810.666	2	6.92	4	0	1	0	1	0	1	.006	2
308		min	-2866.368	3	1.626	15	0	1	0	1	0	1	-.003	3
309	3	max	2810.496	2	6.159	4	0	1	0	1	0	1	.004	2
310		min	-2866.495	3	1.447	15	0	1	0	1	0	1	-.005	3
311	4	max	2810.325	2	5.398	4	0	1	0	1	0	1	.002	2



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2866.623	3	1.268	15	0	1	0	1	0	1	-.006	3
313	5	max	2810.155	2	4.637	4	0	1	0	1	0	1	0	2
314		min	-2866.751	3	1.089	15	0	1	0	1	0	1	-.007	3
315	6	max	2809.985	2	3.876	4	0	1	0	1	0	1	-.001	15
316		min	-2866.879	3	.91	15	0	1	0	1	0	1	-.007	3
317	7	max	2809.814	2	3.115	4	0	1	0	1	0	1	-.002	15
318		min	-2867.006	3	.731	15	0	1	0	1	0	1	-.008	3
319	8	max	2809.644	2	2.381	2	0	1	0	1	0	1	-.002	15
320		min	-2867.134	3	.453	12	0	1	0	1	0	1	-.008	4
321	9	max	2809.474	2	1.788	2	0	1	0	1	0	1	-.002	15
322		min	-2867.262	3	.156	12	0	1	0	1	0	1	-.009	4
323	10	max	2809.303	2	1.195	2	0	1	0	1	0	1	-.002	15
324		min	-2867.39	3	-.256	3	0	1	0	1	0	1	-.01	4
325	11	max	2809.133	2	.602	2	0	1	0	1	0	1	-.002	15
326		min	-2867.517	3	-.701	3	0	1	0	1	0	1	-.01	4
327	12	max	2808.963	2	.009	2	0	1	0	1	0	1	-.002	15
328		min	-2867.645	3	-1.145	3	0	1	0	1	0	1	-.01	4
329	13	max	2808.792	2	-.342	15	0	1	0	1	0	1	-.002	15
330		min	-2867.773	3	-1.59	3	0	1	0	1	0	1	-.009	4
331	14	max	2808.622	2	-.521	15	0	1	0	1	0	1	-.002	15
332		min	-2867.901	3	-2.211	4	0	1	0	1	0	1	-.009	4
333	15	max	2808.451	2	-.7	15	0	1	0	1	0	1	-.002	15
334		min	-2868.028	3	-2.972	4	0	1	0	1	0	1	-.007	4
335	16	max	2808.281	2	-.879	15	0	1	0	1	0	1	-.001	15
336		min	-2868.156	3	-3.733	4	0	1	0	1	0	1	-.006	4
337	17	max	2808.111	2	-1.058	15	0	1	0	1	0	1	-.001	15
338		min	-2868.284	3	-4.494	4	0	1	0	1	0	1	-.004	4
339	18	max	2807.94	2	-1.237	15	0	1	0	1	0	1	0	15
340		min	-2868.412	3	-5.255	4	0	1	0	1	0	1	-.002	4
341	19	max	2807.77	2	-1.415	15	0	1	0	1	0	1	0	1
342		min	-2868.54	3	-6.016	4	0	1	0	1	0	1	0	1
343	M8	1	max	1834.662	2	0	1	0	1	0	1	0	1	1
344		min	68.105	15	0	1	0	1	0	1	0	1	0	1
345	2	max	1834.832	2	0	1	0	1	0	1	0	1	0	1
346		min	68.157	15	0	1	0	1	0	1	0	1	0	1
347	3	max	1835.002	2	0	1	0	1	0	1	0	1	0	1
348		min	68.208	15	0	1	0	1	0	1	0	1	0	1
349	4	max	1835.173	2	0	1	0	1	0	1	0	1	0	1
350		min	68.26	15	0	1	0	1	0	1	0	1	0	1
351	5	max	1835.343	2	0	1	0	1	0	1	0	1	0	1
352		min	68.311	15	0	1	0	1	0	1	0	1	0	1
353	6	max	1835.513	2	0	1	0	1	0	1	0	1	0	1
354		min	68.362	15	0	1	0	1	0	1	0	1	0	1
355	7	max	1835.684	2	0	1	0	1	0	1	0	1	0	1
356		min	68.414	15	0	1	0	1	0	1	0	1	0	1
357	8	max	1835.854	2	0	1	0	1	0	1	0	1	0	1
358		min	68.465	15	0	1	0	1	0	1	0	1	0	1
359	9	max	1836.024	2	0	1	0	1	0	1	0	1	0	1
360		min	68.517	15	0	1	0	1	0	1	0	1	0	1
361	10	max	1836.195	2	0	1	0	1	0	1	0	1	0	1
362		min	68.568	15	0	1	0	1	0	1	0	1	0	1
363	11	max	1836.365	2	0	1	0	1	0	1	0	1	0	1
364		min	68.619	15	0	1	0	1	0	1	0	1	0	1
365	12	max	1836.535	2	0	1	0	1	0	1	0	1	0	1
366		min	68.671	15	0	1	0	1	0	1	0	1	0	1
367	13	max	1836.706	2	0	1	0	1	0	1	0	1	0	1
368		min	68.722	15	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	1836.876	2	0	1	0	1	0	1	0	1	0	1
370			min	68.773	15	0	1	0	1	0	1	0	1	0	1
371		15	max	1837.046	2	0	1	0	1	0	1	0	1	0	1
372			min	68.825	15	0	1	0	1	0	1	0	1	0	1
373		16	max	1837.217	2	0	1	0	1	0	1	0	1	0	1
374			min	68.876	15	0	1	0	1	0	1	0	1	0	1
375		17	max	1837.387	2	0	1	0	1	0	1	0	1	0	1
376			min	68.928	15	0	1	0	1	0	1	0	1	0	1
377		18	max	1837.557	2	0	1	0	1	0	1	0	1	0	1
378			min	68.979	15	0	1	0	1	0	1	0	1	0	1
379		19	max	1837.728	2	0	1	0	1	0	1	0	1	0	1
380			min	69.03	15	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	947.831	2	2.02	4	-0.09	15	0	1	0	2	0	1
382			min	-1382.186	3	.475	15	-.165	1	0	3	0	3	0	1
383		2	max	948.352	2	1.901	4	-.009	15	0	1	0	10	0	15
384			min	-1381.795	3	.447	15	-.165	1	0	3	0	1	0	4
385		3	max	948.873	2	1.782	4	-.009	15	0	1	0	15	0	15
386			min	-1381.405	3	.419	15	-.165	1	0	3	0	1	-.001	4
387		4	max	949.393	2	1.663	4	-.009	15	0	1	0	15	0	15
388			min	-1381.014	3	.391	15	-.165	1	0	3	0	1	-.002	4
389		5	max	949.914	2	1.544	4	-.009	15	0	1	0	15	0	15
390			min	-1380.624	3	.363	15	-.165	1	0	3	0	1	-.003	4
391		6	max	950.435	2	1.425	4	-.009	15	0	1	0	15	0	15
392			min	-1380.233	3	.335	15	-.165	1	0	3	0	1	-.003	4
393		7	max	950.956	2	1.306	4	-.009	15	0	1	0	15	0	15
394			min	-1379.843	3	.307	15	-.165	1	0	3	0	1	-.004	4
395		8	max	951.476	2	1.187	4	-.009	15	0	1	0	15	0	15
396			min	-1379.452	3	.279	15	-.165	1	0	3	0	1	-.004	4
397		9	max	951.997	2	1.069	4	-.009	15	0	1	0	15	-.001	15
398			min	-1379.062	3	.242	12	-.165	1	0	3	0	1	-.004	4
399		10	max	952.518	2	.95	4	-.009	15	0	1	0	15	-.001	15
400			min	-1378.671	3	.196	12	-.165	1	0	3	0	1	-.005	4
401		11	max	953.038	2	.852	2	-.009	15	0	1	0	15	-.001	15
402			min	-1378.28	3	.15	12	-.165	1	0	3	0	1	-.005	4
403		12	max	953.559	2	.759	2	-.009	15	0	1	0	15	-.001	15
404			min	-1377.89	3	.104	12	-.165	1	0	3	0	1	-.005	4
405		13	max	954.08	2	.666	2	-.009	15	0	1	0	15	-.001	15
406			min	-1377.499	3	.057	12	-.165	1	0	3	0	1	-.006	4
407		14	max	954.6	2	.574	2	-.009	15	0	1	0	15	-.001	15
408			min	-1377.109	3	-.001	3	-.165	1	0	3	0	1	-.006	4
409		15	max	955.121	2	.481	2	-.009	15	0	1	0	15	-.001	15
410			min	-1376.718	3	-.071	3	-.165	1	0	3	0	1	-.006	4
411		16	max	955.642	2	.389	2	-.009	15	0	1	0	15	-.001	15
412			min	-1376.328	3	-.14	3	-.165	1	0	3	0	1	-.006	4
413		17	max	956.163	2	.296	2	-.009	15	0	1	0	15	-.001	12
414			min	-1375.937	3	-.21	3	-.165	1	0	3	0	1	-.006	4
415		18	max	956.683	2	.203	2	-.009	15	0	1	0	15	-.001	12
416			min	-1375.547	3	-.279	3	-.165	1	0	3	0	1	-.006	4
417		19	max	957.204	2	.111	2	-.009	15	0	1	0	15	-.001	12
418			min	-1375.156	3	-.348	3	-.165	1	0	3	-.001	1	-.006	4
419	M11	1	max	856.557	2	7.663	4	-.01	15	0	1	0	15	.006	4
420			min	-951.878	3	1.801	15	-.169	1	0	3	0	1	.001	12
421		2	max	856.386	2	6.902	4	-.01	15	0	1	0	15	.004	2
422			min	-952.006	3	1.623	15	-.169	1	0	3	0	1	0	3
423		3	max	856.216	2	6.141	4	-.01	15	0	1	0	15	.001	2
424			min	-952.134	3	1.444	15	-.169	1	0	3	0	1	-.001	3
425		4	max	856.046	2	5.38	4	-.01	15	0	1	0	15	0	15



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-952.262	3	1.265	15	-.169	1	0	3	0	1	-.003	3
427		5	max	855.875	2	4.619	4	-.01	15	0	1	0	15	0	15
428			min	-952.389	3	1.086	15	-.169	1	0	3	0	1	-.004	4
429		6	max	855.705	2	3.858	4	-.01	15	0	1	0	15	-.001	15
430			min	-952.517	3	.907	15	-.169	1	0	3	0	1	-.006	4
431		7	max	855.535	2	3.097	4	-.01	15	0	1	0	15	-.002	15
432			min	-952.645	3	.728	15	-.169	1	0	3	0	1	-.007	4
433		8	max	855.364	2	2.336	4	-.01	15	0	1	0	15	-.002	15
434			min	-952.773	3	.549	15	-.169	1	0	3	0	1	-.008	4
435		9	max	855.194	2	1.575	4	-.01	15	0	1	0	15	-.002	15
436			min	-.952.9	3	.37	15	-.169	1	0	3	0	1	-.009	4
437		10	max	855.024	2	.814	4	-.01	15	0	1	0	15	-.002	15
438			min	-953.028	3	.168	12	-.169	1	0	3	0	1	-.01	4
439		11	max	854.853	2	.216	2	-.01	15	0	1	0	15	-.002	15
440			min	-953.156	3	-.208	3	-.169	1	0	3	0	1	-.01	4
441		12	max	854.683	2	-.166	15	-.01	15	0	1	0	15	-.002	15
442			min	-953.284	3	-.708	4	-.169	1	0	3	0	1	-.01	4
443		13	max	854.513	2	-.345	15	-.01	15	0	1	0	15	-.002	15
444			min	-953.412	3	-1.469	4	-.169	1	0	3	-.001	1	-.009	4
445		14	max	854.342	2	-.524	15	-.01	15	0	1	0	15	-.002	15
446			min	-953.539	3	-2.23	4	-.169	1	0	3	-.001	1	-.009	4
447		15	max	854.172	2	-.703	15	-.01	15	0	1	0	15	-.002	15
448			min	-953.667	3	-2.991	4	-.169	1	0	3	-.001	1	-.008	4
449		16	max	854.002	2	-.882	15	-.01	15	0	1	0	15	-.001	15
450			min	-953.795	3	-3.752	4	-.169	1	0	3	-.001	1	-.006	4
451		17	max	853.831	2	-1.061	15	-.01	15	0	1	0	15	-.001	15
452			min	-953.923	3	-4.513	4	-.169	1	0	3	-.001	1	-.004	4
453		18	max	853.661	2	-1.239	15	-.01	15	0	1	0	15	0	15
454			min	-954.05	3	-5.274	4	-.169	1	0	3	-.001	1	-.002	4
455		19	max	853.491	2	-1.418	15	-.01	15	0	1	0	15	0	1
456			min	-954.178	3	-6.035	4	-.169	1	0	3	-.001	1	0	1
457	M12	1	max	711.046	1	0	1	5.707	1	0	1	0	15	0	1
458			min	33.373	15	0	1	.325	15	0	1	-.001	1	0	1
459		2	max	711.216	1	0	1	5.707	1	0	1	0	15	0	1
460			min	33.425	15	0	1	.325	15	0	1	0	1	0	1
461		3	max	711.387	1	0	1	5.707	1	0	1	0	10	0	1
462			min	33.476	15	0	1	.325	15	0	1	0	1	0	1
463		4	max	711.557	1	0	1	5.707	1	0	1	0	1	0	1
464			min	33.527	15	0	1	.325	15	0	1	0	15	0	1
465		5	max	711.727	1	0	1	5.707	1	0	1	.001	1	0	1
466			min	33.579	15	0	1	.325	15	0	1	0	15	0	1
467		6	max	711.898	1	0	1	5.707	1	0	1	.002	1	0	1
468			min	33.63	15	0	1	.325	15	0	1	0	15	0	1
469		7	max	712.068	1	0	1	5.707	1	0	1	.003	1	0	1
470			min	33.682	15	0	1	.325	15	0	1	0	15	0	1
471		8	max	712.238	1	0	1	5.707	1	0	1	.003	1	0	1
472			min	33.733	15	0	1	.325	15	0	1	0	15	0	1
473		9	max	712.409	1	0	1	5.707	1	0	1	.004	1	0	1
474			min	33.784	15	0	1	.325	15	0	1	0	15	0	1
475		10	max	712.579	1	0	1	5.707	1	0	1	.005	1	0	1
476			min	33.836	15	0	1	.325	15	0	1	0	15	0	1
477		11	max	712.749	1	0	1	5.707	1	0	1	.005	1	0	1
478			min	33.887	15	0	1	.325	15	0	1	0	15	0	1
479		12	max	712.92	1	0	1	5.707	1	0	1	.006	1	0	1
480			min	33.939	15	0	1	.325	15	0	1	0	15	0	1
481		13	max	713.09	1	0	1	5.707	1	0	1	.006	1	0	1
482			min	33.99	15	0	1	.325	15	0	1	0	15	0	1



Company : Schletter, Inc.  
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Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483		14	max	713.261	1	0	1	5.707	1	0	1	.007	1	0	1
484			min	34.041	15	0	1	.325	15	0	1	0	15	0	1
485		15	max	713.431	1	0	1	5.707	1	0	1	.008	1	0	1
486			min	34.093	15	0	1	.325	15	0	1	0	15	0	1
487		16	max	713.601	1	0	1	5.707	1	0	1	.008	1	0	1
488			min	34.144	15	0	1	.325	15	0	1	0	15	0	1
489		17	max	713.772	1	0	1	5.707	1	0	1	.009	1	0	1
490			min	34.196	15	0	1	.325	15	0	1	0	15	0	1
491		18	max	713.942	1	0	1	5.707	1	0	1	.01	1	0	1
492			min	34.247	15	0	1	.325	15	0	1	0	15	0	1
493		19	max	714.112	1	0	1	5.707	1	0	1	.01	1	0	1
494			min	34.298	15	0	1	.325	15	0	1	0	15	0	1
495	M1	1	max	121.899	1	706.966	3	-2.828	15	0	2	.117	1	0	15
496			min	6.821	15	-369.071	2	-49.136	1	0	3	.007	15	-.012	2
497		2	max	122.721	1	706.086	3	-2.828	15	0	2	.091	1	.183	2
498			min	7.069	15	-370.245	2	-49.136	1	0	3	.005	15	-.375	3
499		3	max	597.993	3	484.981	2	-2.818	15	0	3	.065	1	.368	2
500			min	-344.91	2	-552.387	3	-49.027	1	0	2	.004	15	-.732	3
501		4	max	598.609	3	483.808	2	-2.818	15	0	3	.039	1	.112	2
502			min	-344.089	2	-553.267	3	-49.027	1	0	2	.002	15	-.44	3
503		5	max	599.226	3	482.634	2	-2.818	15	0	3	.013	1	-.003	15
504			min	-343.267	2	-554.147	3	-49.027	1	0	2	0	15	-.148	3
505		6	max	599.842	3	481.461	2	-2.818	15	0	3	0	15	.145	3
506			min	-342.445	2	-555.027	3	-49.027	1	0	2	-.013	1	-.397	2
507		7	max	600.458	3	480.288	2	-2.818	15	0	3	-.002	15	.438	3
508			min	-341.624	2	-555.907	3	-49.027	1	0	2	-.039	1	-.651	2
509		8	max	601.074	3	479.114	2	-2.818	15	0	3	-.004	15	.731	3
510			min	-340.802	2	-556.787	3	-49.027	1	0	2	-.064	1	-.904	2
511		9	max	616.115	3	52.433	2	-4.455	15	0	9	.041	1	.849	3
512			min	-286.669	2	.359	15	-77.726	1	0	3	.002	15	-1.033	2
513		10	max	616.732	3	51.26	2	-4.455	15	0	9	0	10	.832	3
514			min	-285.848	2	.005	15	-77.726	1	0	3	0	1	-1.061	2
515		11	max	617.348	3	50.086	2	-4.455	15	0	9	-.002	15	.815	3
516			min	-285.026	2	-1.45	4	-77.726	1	0	3	-.041	1	-1.087	2
517		12	max	632.078	3	384.019	3	-2.755	15	0	2	.064	1	.715	3
518			min	-230.746	2	-589.775	2	-48.267	1	0	3	.004	15	-.966	2
519		13	max	632.694	3	383.139	3	-2.755	15	0	2	.038	1	.512	3
520			min	-229.924	2	-590.948	2	-48.267	1	0	3	.002	15	-.655	2
521		14	max	633.31	3	382.259	3	-2.755	15	0	2	.013	1	.31	3
522			min	-229.103	2	-592.122	2	-48.267	1	0	3	0	15	-.343	2
523		15	max	633.926	3	381.379	3	-2.755	15	0	2	0	15	.109	3
524			min	-228.281	2	-593.295	2	-48.267	1	0	3	-.013	1	-.039	1
525		16	max	634.543	3	380.499	3	-2.755	15	0	2	-.002	15	.283	2
526			min	-227.459	2	-594.469	2	-48.267	1	0	3	-.038	1	-.092	3
527		17	max	635.159	3	379.619	3	-2.755	15	0	2	-.004	15	.597	2
528			min	-226.638	2	-595.642	2	-48.267	1	0	3	-.063	1	-.293	3
529		18	max	-7.077	15	586.071	2	-3.071	15	0	3	-.005	15	.302	2
530			min	-123.14	1	-295.966	3	-53.717	1	0	2	-.09	1	-.145	3
531		19	max	-6.829	15	584.898	2	-3.071	15	0	3	-.007	15	.011	3
532			min	-122.319	1	-296.846	3	-53.717	1	0	2	-.118	1	-.007	2
533	M5	1	max	274.913	1	2337.771	3	0	1	0	1	0	1	.025	2
534			min	10.398	12	-1278.202	2	0	1	0	1	0	1	0	15
535		2	max	275.734	1	2336.891	3	0	1	0	1	0	1	.7	2
536			min	10.809	12	-1279.375	2	0	1	0	1	0	1	-1.23	3
537		3	max	1841.372	3	1322.053	2	0	1	0	1	0	1	1.344	2
538			min	-1093.649	2	-1630.552	3	0	1	0	1	0	1	-2.415	3
539		4	max	1841.988	3	1320.88	2	0	1	0	1	0	1	.646	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-1092.827	2	-1631.432	3	0	1	0	1	0	1	-1.554	3
541		5	max	1842.604	3	1319.706	2	0	1	0	1	0	1	.015	9
542			min	-1092.006	2	-1632.312	3	0	1	0	1	0	1	-.693	3
543		6	max	1843.22	3	1318.533	2	0	1	0	1	0	1	.168	3
544			min	-1091.184	2	-1633.192	3	0	1	0	1	0	1	-.746	2
545		7	max	1843.837	3	1317.359	2	0	1	0	1	0	1	1.03	3
546			min	-1090.363	2	-1634.072	3	0	1	0	1	0	1	-1.442	2
547		8	max	1844.453	3	1316.186	2	0	1	0	1	0	1	1.893	3
548			min	-1089.541	2	-1634.952	3	0	1	0	1	0	1	-2.137	2
549		9	max	1859.25	3	177.875	2	0	1	0	1	0	1	2.175	3
550			min	-967.857	2	.35	15	0	1	0	1	0	1	-2.447	2
551		10	max	1859.866	3	176.701	2	0	1	0	1	0	1	2.108	3
552			min	-967.035	2	-.004	15	0	1	0	1	0	1	-2.54	2
553		11	max	1860.482	3	175.528	2	0	1	0	1	0	1	2.042	3
554			min	-966.213	2	-1.433	4	0	1	0	1	0	1	-2.633	2
555		12	max	1875.902	3	1095.813	3	0	1	0	1	0	1	1.791	3
556			min	-844.824	2	-1673.989	2	0	1	0	1	0	1	-2.361	2
557		13	max	1876.518	3	1094.933	3	0	1	0	1	0	1	1.213	3
558			min	-844.002	2	-1675.162	2	0	1	0	1	0	1	-1.478	2
559		14	max	1877.135	3	1094.053	3	0	1	0	1	0	1	.636	3
560			min	-843.181	2	-1676.335	2	0	1	0	1	0	1	-.593	2
561		15	max	1877.751	3	1093.172	3	0	1	0	1	0	1	.291	2
562			min	-842.359	2	-1677.509	2	0	1	0	1	0	1	0	13
563		16	max	1878.367	3	1092.292	3	0	1	0	1	0	1	1.177	2
564			min	-841.538	2	-1678.682	2	0	1	0	1	0	1	-.518	3
565		17	max	1878.983	3	1091.412	3	0	1	0	1	0	1	2.063	2
566			min	-840.716	2	-1679.856	2	0	1	0	1	0	1	-1.094	3
567		18	max	-12.1	12	1983.564	2	0	1	0	1	0	1	1.06	2
568			min	-274.896	1	-1039.258	3	0	1	0	1	0	1	-.571	3
569		19	max	-11.689	12	1982.391	2	0	1	0	1	0	1	.014	2
570			min	-274.075	1	-1040.138	3	0	1	0	1	0	1	-.023	3
571	M9	1	max	121.899	1	706.966	3	49.136	1	0	3	-.007	15	0	15
572			min	6.821	15	-369.071	2	2.828	15	0	2	-.117	1	-.012	2
573		2	max	122.721	1	706.086	3	49.136	1	0	3	-.005	15	.183	2
574			min	7.069	15	-370.245	2	2.828	15	0	2	-.091	1	-.375	3
575		3	max	597.993	3	484.981	2	49.027	1	0	2	-.004	15	.368	2
576			min	-344.91	2	-552.387	3	2.818	15	0	3	-.065	1	-.732	3
577		4	max	598.609	3	483.808	2	49.027	1	0	2	-.002	15	.112	2
578			min	-344.089	2	-553.267	3	2.818	15	0	3	-.039	1	-.44	3
579		5	max	599.226	3	482.634	2	49.027	1	0	2	0	15	-.003	15
580			min	-343.267	2	-554.147	3	2.818	15	0	3	-.013	1	-.148	3
581		6	max	599.842	3	481.461	2	49.027	1	0	2	.013	1	.145	3
582			min	-342.445	2	-555.027	3	2.818	15	0	3	0	15	-.397	2
583		7	max	600.458	3	480.288	2	49.027	1	0	2	.039	1	.438	3
584			min	-341.624	2	-555.907	3	2.818	15	0	3	.002	15	-.651	2
585		8	max	601.074	3	479.114	2	49.027	1	0	2	.064	1	.731	3
586			min	-340.802	2	-556.787	3	2.818	15	0	3	.004	15	-.904	2
587		9	max	616.115	3	52.433	2	77.726	1	0	3	-.002	15	.849	3
588			min	-286.669	2	.359	15	4.455	15	0	9	-.041	1	-1.033	2
589		10	max	616.732	3	51.26	2	77.726	1	0	3	0	1	.832	3
590			min	-285.848	2	.005	15	4.455	15	0	9	0	10	-1.061	2
591		11	max	617.348	3	50.086	2	77.726	1	0	3	.041	1	.815	3
592			min	-285.026	2	-1.45	4	4.455	15	0	9	.002	15	-1.087	2
593		12	max	632.078	3	384.019	3	48.267	1	0	3	-.004	15	.715	3
594			min	-230.746	2	-589.775	2	2.755	15	0	2	-.064	1	-.966	2
595		13	max	632.694	3	383.139	3	48.267	1	0	3	-.002	15	.512	3
596			min	-229.924	2	-590.948	2	2.755	15	0	2	-.038	1	-.655	2



Company : Schletter, Inc.  
Designer : HCV  
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Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	633.31	3	382.259	3	48.267	1	0	3	0	15	.31	3
598		min	-229.103	2	-592.122	2	2.755	15	0	2	-.013	1	-.343	2
599	15	max	633.926	3	381.379	3	48.267	1	0	3	.013	1	.109	3
600		min	-228.281	2	-593.295	2	2.755	15	0	2	0	15	-.039	1
601	16	max	634.543	3	380.499	3	48.267	1	0	3	.038	1	.283	2
602		min	-227.459	2	-594.469	2	2.755	15	0	2	.002	15	-.092	3
603	17	max	635.159	3	379.619	3	48.267	1	0	3	.063	1	.597	2
604		min	-226.638	2	-595.642	2	2.755	15	0	2	.004	15	-.293	3
605	18	max	-7.077	15	586.071	2	53.717	1	0	2	.09	1	.302	2
606		min	-123.14	1	-295.966	3	3.071	15	0	3	.005	15	-.145	3
607	19	max	-6.829	15	584.898	2	53.717	1	0	2	.118	1	.011	3
608		min	-122.319	1	-296.846	3	3.071	15	0	3	.007	15	-.007	2

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.112	2	.01	3	9.512e-3	2	NC	1	NC	1
2			min	0	15	-.031	3	-.006	2	-3.06e-3	3	NC	1	NC	1
3		2	max	0	1	.088	3	.012	3	1.036e-2	2	NC	4	NC	1
4			min	0	15	.001	15	-.003	10	-2.939e-3	3	1411.468	3	NC	1
5		3	max	0	1	.185	3	.023	1	1.122e-2	2	NC	4	NC	2
6			min	0	15	0	9	-.002	10	-2.818e-3	3	777.087	3	6906.683	1
7		4	max	0	1	.246	3	.035	1	1.207e-2	2	NC	4	NC	2
8			min	0	15	-.005	9	-.001	10	-2.696e-3	3	606.988	3	4711.209	1
9		5	max	0	1	.263	3	.04	1	1.292e-2	2	NC	4	NC	2
10			min	0	15	-.005	9	-.002	10	-2.575e-3	3	571.747	3	4124.064	1
11		6	max	0	1	.237	3	.037	1	1.377e-2	2	NC	4	NC	2
12			min	0	15	0	15	-.004	10	-2.454e-3	3	626.37	3	4423.746	1
13		7	max	0	1	.178	3	.028	3	1.463e-2	2	NC	4	NC	2
14			min	0	15	.001	15	-.006	10	-2.332e-3	3	805.652	3	6007.605	1
15		8	max	0	1	.135	2	.03	3	1.548e-2	2	NC	1	NC	1
16			min	0	15	.002	15	-.011	2	-2.211e-3	3	1280.632	3	8652.928	3
17		9	max	0	1	.178	2	.03	3	1.633e-2	2	NC	4	NC	1
18			min	0	15	.003	15	-.018	2	-2.089e-3	3	2544.55	2	8411.001	3
19		10	max	0	1	.197	2	.03	3	1.718e-2	2	NC	4	NC	1
20			min	0	1	-.003	3	-.022	2	-1.968e-3	3	1971.306	2	8371.587	3
21		11	max	0	15	.178	2	.03	3	1.633e-2	2	NC	4	NC	1
22			min	0	1	.003	15	-.018	2	-2.089e-3	3	2544.55	2	8411.001	3
23		12	max	0	15	.135	2	.03	3	1.548e-2	2	NC	1	NC	1
24			min	0	1	.002	15	-.011	2	-2.211e-3	3	1280.632	3	8652.928	3
25		13	max	0	15	.178	3	.028	3	1.463e-2	2	NC	4	NC	2
26			min	0	1	.001	15	-.006	10	-2.332e-3	3	805.652	3	6007.605	1
27		14	max	0	15	.237	3	.037	1	1.377e-2	2	NC	4	NC	2
28			min	0	1	0	15	-.004	10	-2.454e-3	3	626.37	3	4423.746	1
29		15	max	0	15	.263	3	.04	1	1.292e-2	2	NC	4	NC	2
30			min	0	1	-.005	9	-.002	10	-2.575e-3	3	571.747	3	4124.064	1
31		16	max	0	15	.246	3	.035	1	1.207e-2	2	NC	4	NC	2
32			min	0	1	-.005	9	-.001	10	-2.696e-3	3	606.988	3	4711.209	1
33		17	max	0	15	.185	3	.023	1	1.122e-2	2	NC	4	NC	2
34			min	0	1	0	9	-.002	10	-2.818e-3	3	777.087	3	6906.683	1
35		18	max	0	15	.088	3	.012	3	1.036e-2	2	NC	4	NC	1
36			min	0	1	.001	15	-.003	10	-2.939e-3	3	1411.468	3	NC	1
37		19	max	0	15	.112	2	.01	3	9.512e-3	2	NC	1	NC	1
38			min	0	1	-.031	3	-.006	2	-3.06e-3	3	NC	1	NC	1
39	M14	1	max	0	1	.265	3	.009	3	5.217e-3	2	NC	1	NC	1
40			min	0	15	-.353	2	-.006	2	-4.447e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41		2	max	0	1	.416	3	.01	3	6.022e-3	2	NC	4	NC	1
42			min	0	15	-.49	2	-.003	2	-5.204e-3	3	1110.529	3	NC	1
43		3	max	0	1	.549	3	.018	1	6.827e-3	2	NC	5	NC	2
44			min	0	15	-.613	2	-.002	10	-5.961e-3	3	591.189	3	9091.094	1
45		4	max	0	1	.651	3	.028	1	7.632e-3	2	NC	5	NC	2
46			min	0	15	-.713	2	-.002	10	-6.717e-3	3	435.438	3	5732.263	1
47		5	max	0	1	.714	3	.034	1	8.437e-3	2	NC	5	NC	2
48			min	0	15	-.785	2	-.002	10	-7.474e-3	3	373.851	3	4801.781	1
49		6	max	0	1	.739	3	.033	1	9.242e-3	2	NC	5	NC	2
50			min	0	15	-.826	2	-.003	10	-8.231e-3	3	354.404	3	5005.389	1
51		7	max	0	1	.73	3	.025	3	1.005e-2	2	NC	5	NC	2
52			min	0	15	-.84	2	-.006	10	-8.988e-3	3	345.058	2	6654.344	1
53		8	max	0	1	.698	3	.026	3	1.085e-2	2	NC	5	NC	1
54			min	0	15	-.833	2	-.01	2	-9.745e-3	3	349.803	2	9817.861	3
55		9	max	0	1	.662	3	.027	3	1.166e-2	2	NC	5	NC	1
56			min	0	15	-.817	2	-.017	2	-1.05e-2	3	361.652	2	9487.506	3
57		10	max	0	1	.643	3	.027	3	1.246e-2	2	NC	5	NC	1
58			min	0	1	-.808	2	-.02	2	-1.126e-2	3	369.175	2	9426.223	3
59		11	max	0	15	.662	3	.027	3	1.166e-2	2	NC	5	NC	1
60			min	0	1	-.817	2	-.017	2	-1.05e-2	3	361.652	2	9487.506	3
61		12	max	0	15	.698	3	.026	3	1.085e-2	2	NC	5	NC	1
62			min	0	1	-.833	2	-.01	2	-9.745e-3	3	349.803	2	9817.861	3
63		13	max	0	15	.73	3	.025	3	1.005e-2	2	NC	5	NC	2
64			min	0	1	-.84	2	-.006	10	-8.988e-3	3	345.058	2	6654.344	1
65		14	max	0	15	.739	3	.033	1	9.242e-3	2	NC	5	NC	2
66			min	0	1	-.826	2	-.003	10	-8.231e-3	3	354.404	3	5005.389	1
67		15	max	0	15	.714	3	.034	1	8.437e-3	2	NC	5	NC	2
68			min	0	1	-.785	2	-.002	10	-7.474e-3	3	373.851	3	4801.781	1
69		16	max	0	15	.651	3	.028	1	7.632e-3	2	NC	5	NC	2
70			min	0	1	-.713	2	-.002	10	-6.717e-3	3	435.438	3	5732.263	1
71		17	max	0	15	.549	3	.018	1	6.827e-3	2	NC	5	NC	2
72			min	0	1	-.613	2	-.002	10	-5.961e-3	3	591.189	3	9091.094	1
73		18	max	0	15	.416	3	.01	3	6.022e-3	2	NC	4	NC	1
74			min	0	1	-.49	2	-.003	2	-5.204e-3	3	1110.529	3	NC	1
75		19	max	0	15	.265	3	.009	3	5.217e-3	2	NC	1	NC	1
76			min	0	1	-.353	2	-.006	2	-4.447e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.269	3	.008	3	3.958e-3	3	NC	1	NC	1
78			min	0	1	-.352	2	-.005	2	-5.492e-3	2	NC	1	NC	1
79		2	max	0	15	.382	3	.01	3	4.633e-3	3	NC	4	NC	1
80			min	0	1	-.523	2	-.003	10	-6.347e-3	2	980.228	2	NC	1
81		3	max	0	15	.484	3	.018	1	5.308e-3	3	NC	5	NC	2
82			min	0	1	-.674	2	-.002	10	-7.203e-3	2	520.674	2	9045.016	1
83		4	max	0	15	.567	3	.029	1	5.984e-3	3	NC	5	NC	2
84			min	0	1	-.791	2	-.001	10	-8.058e-3	2	382.097	2	5703.551	1
85		5	max	0	15	.627	3	.034	1	6.659e-3	3	NC	5	NC	2
86			min	0	1	-.866	2	-.001	10	-8.914e-3	2	326.306	2	4774.341	1
87		6	max	0	15	.662	3	.033	1	7.335e-3	3	NC	5	NC	2
88			min	0	1	-.899	2	-.003	10	-9.769e-3	2	307.064	2	4968.127	1
89		7	max	0	15	.675	3	.025	1	8.01e-3	3	NC	5	NC	2
90			min	0	1	-.893	2	-.005	10	-1.062e-2	2	309.998	2	6577.113	1
91		8	max	0	15	.671	3	.024	3	8.685e-3	3	NC	5	NC	1
92			min	0	1	-.863	2	-.009	2	-1.148e-2	2	328.561	2	NC	1
93		9	max	0	15	.659	3	.025	3	9.361e-3	3	NC	5	NC	1
94			min	0	1	-.825	2	-.016	2	-1.234e-2	2	354.581	2	NC	1
95		10	max	0	1	.651	3	.025	3	1.004e-2	3	NC	5	NC	1
96			min	0	1	-.806	2	-.019	2	-1.319e-2	2	369.687	2	NC	1
97		11	max	0	1	.659	3	.025	3	9.361e-3	3	NC	5	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-.825	2	-.016	2	-1.234e-2	2	354.581	2	NC	1
99		max	0	1	.671	3	.024	3	8.685e-3	3	NC	5	NC	1
100		min	0	15	-.863	2	-.009	2	-1.148e-2	2	328.561	2	NC	1
101		max	0	1	.675	3	.025	1	8.01e-3	3	NC	5	NC	2
102		min	0	15	-.893	2	-.005	10	-1.062e-2	2	309.998	2	6577.113	1
103		max	0	1	.662	3	.033	1	7.335e-3	3	NC	5	NC	2
104		min	0	15	-.899	2	-.003	10	-9.769e-3	2	307.064	2	4968.127	1
105		max	0	1	.627	3	.034	1	6.659e-3	3	NC	5	NC	2
106		min	0	15	-.866	2	-.001	10	-8.914e-3	2	326.306	2	4774.341	1
107		max	0	1	.567	3	.029	1	5.984e-3	3	NC	5	NC	2
108		min	0	15	-.791	2	-.001	10	-8.058e-3	2	382.097	2	5703.551	1
109		max	0	1	.484	3	.018	1	5.308e-3	3	NC	5	NC	2
110		min	0	15	-.674	2	-.002	10	-7.203e-3	2	520.674	2	9045.016	1
111		max	0	1	.382	3	.01	3	4.633e-3	3	NC	4	NC	1
112		min	0	15	-.523	2	-.003	10	-6.347e-3	2	980.228	2	NC	1
113		max	0	1	.269	3	.008	3	3.958e-3	3	NC	1	NC	1
114		min	0	15	-.352	2	-.005	2	-5.492e-3	2	NC	1	NC	1
115	M16	max	0	15	.099	2	.007	3	7.304e-3	3	NC	1	NC	1
116		min	0	1	-.09	3	-.005	2	-7.811e-3	2	NC	1	NC	1
117		max	0	15	.02	1	.01	1	8.083e-3	3	NC	4	NC	1
118		min	0	1	-.055	3	-.002	10	-8.284e-3	2	1947.273	2	NC	1
119		max	0	15	.003	4	.024	1	8.862e-3	3	NC	4	NC	2
120		min	0	1	-.056	2	0	10	-8.757e-3	2	1087.113	2	6876.464	1
121		max	0	15	.001	13	.035	1	9.642e-3	3	NC	4	NC	2
122		min	0	1	-.094	2	0	10	-9.23e-3	2	871.519	2	4670.652	1
123		max	0	15	.001	13	.04	1	1.042e-2	3	NC	4	NC	2
124		min	0	1	-.096	2	0	10	-9.703e-3	2	859.458	2	4067.23	1
125		max	0	15	.004	9	.038	1	1.12e-2	3	NC	4	NC	2
126		min	0	1	-.065	2	-.002	10	-1.018e-2	2	1026.289	2	4326.498	1
127		max	0	15	.014	9	.028	1	1.198e-2	3	NC	3	NC	2
128		min	0	1	-.084	3	-.004	10	-1.065e-2	2	1599.742	2	5774.127	1
129		max	0	15	.065	2	.021	3	1.276e-2	3	NC	1	NC	1
130		min	0	1	-.126	3	-.007	10	-1.112e-2	2	4619.138	3	NC	1
131		max	0	15	.129	2	.022	3	1.354e-2	3	NC	4	NC	1
132		min	0	1	-.162	3	-.014	2	-1.16e-2	2	2323.511	3	NC	1
133		max	0	1	.158	2	.021	3	1.432e-2	3	NC	4	NC	1
134		min	0	1	-.178	3	-.017	2	-1.207e-2	2	1905.238	3	NC	1
135		max	0	1	.129	2	.022	3	1.354e-2	3	NC	4	NC	1
136		min	0	15	-.162	3	-.014	2	-1.16e-2	2	2323.511	3	NC	1
137		max	0	1	.065	2	.021	3	1.276e-2	3	NC	1	NC	1
138		min	0	15	-.126	3	-.007	10	-1.112e-2	2	4619.138	3	NC	1
139		max	0	1	.014	9	.028	1	1.198e-2	3	NC	3	NC	2
140		min	0	15	-.084	3	-.004	10	-1.065e-2	2	1599.742	2	5774.127	1
141		max	0	1	.004	9	.038	1	1.12e-2	3	NC	4	NC	2
142		min	0	15	-.065	2	-.002	10	-1.018e-2	2	1026.289	2	4326.498	1
143		max	0	1	.001	13	.04	1	1.042e-2	3	NC	4	NC	2
144		min	0	15	-.096	2	0	10	-9.703e-3	2	859.458	2	4067.23	1
145		max	0	1	.001	13	.035	1	9.642e-3	3	NC	4	NC	2
146		min	0	15	-.094	2	0	10	-9.23e-3	2	871.519	2	4670.652	1
147		max	0	1	.003	4	.024	1	8.862e-3	3	NC	4	NC	2
148		min	0	15	-.056	2	0	10	-8.757e-3	2	1087.113	2	6876.464	1
149		max	0	1	.02	1	.01	1	8.083e-3	3	NC	4	NC	1
150		min	0	15	-.055	3	-.002	10	-8.284e-3	2	1947.273	2	NC	1
151		max	0	1	.099	2	.007	3	7.304e-3	3	NC	1	NC	1
152		min	0	15	-.09	3	-.005	2	-7.811e-3	2	NC	1	NC	1
153	M2	max	.007	2	.01	2	.004	1	-5.942e-6	15	NC	1	NC	1
154		min	-.01	3	-.016	3	0	15	-1.029e-4	1	7453.502	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.007	2	.009	2	.004	1	-5.637e-6	15	NC	1	NC	1
156			min	-.01	3	-.015	3	0	15	-9.758e-5	1	8642.116	2	NC	1
157		3	max	.006	2	.008	2	.003	1	-5.332e-6	15	NC	1	NC	1
158			min	-.009	3	-.015	3	0	15	-9.23e-5	1	NC	1	NC	1
159		4	max	.006	2	.006	2	.003	1	-5.028e-6	15	NC	1	NC	1
160			min	-.009	3	-.014	3	0	15	-8.702e-5	1	NC	1	NC	1
161		5	max	.005	2	.005	2	.003	1	-4.723e-6	15	NC	1	NC	1
162			min	-.008	3	-.014	3	0	15	-8.174e-5	1	NC	1	NC	1
163		6	max	.005	2	.004	2	.002	1	-4.418e-6	15	NC	1	NC	1
164			min	-.007	3	-.013	3	0	15	-7.646e-5	1	NC	1	NC	1
165		7	max	.005	2	.002	2	.002	1	-4.113e-6	15	NC	1	NC	1
166			min	-.007	3	-.012	3	0	15	-7.117e-5	1	NC	1	NC	1
167		8	max	.004	2	.001	2	.002	1	-3.809e-6	15	NC	1	NC	1
168			min	-.006	3	-.012	3	0	15	-6.589e-5	1	NC	1	NC	1
169		9	max	.004	2	0	2	.001	1	-3.504e-6	15	NC	1	NC	1
170			min	-.006	3	-.011	3	0	15	-6.061e-5	1	NC	1	NC	1
171		10	max	.004	2	0	2	.001	1	-3.199e-6	15	NC	1	NC	1
172			min	-.005	3	-.01	3	0	15	-5.533e-5	1	NC	1	NC	1
173		11	max	.003	2	0	2	0	1	-2.894e-6	15	NC	1	NC	1
174			min	-.005	3	-.009	3	0	15	-5.005e-5	1	NC	1	NC	1
175		12	max	.003	2	-.001	15	0	1	-2.59e-6	15	NC	1	NC	1
176			min	-.004	3	-.008	3	0	15	-4.476e-5	1	NC	1	NC	1
177		13	max	.002	2	-.001	15	0	1	-2.285e-6	15	NC	1	NC	1
178			min	-.003	3	-.007	3	0	15	-3.948e-5	1	NC	1	NC	1
179		14	max	.002	2	-.001	15	0	1	-1.98e-6	15	NC	1	NC	1
180			min	-.003	3	-.006	3	0	15	-3.42e-5	1	NC	1	NC	1
181		15	max	.002	2	-.001	15	0	1	-1.675e-6	15	NC	1	NC	1
182			min	-.002	3	-.005	3	0	15	-2.892e-5	1	NC	1	NC	1
183		16	max	.001	2	0	15	0	1	-1.371e-6	15	NC	1	NC	1
184			min	-.002	3	-.004	3	0	15	-2.364e-5	1	NC	1	NC	1
185		17	max	0	2	0	15	0	1	-1.066e-6	15	NC	1	NC	1
186			min	-.001	3	-.003	3	0	15	-1.835e-5	1	NC	1	NC	1
187		18	max	0	2	0	15	0	1	-7.61e-7	15	NC	1	NC	1
188			min	0	3	-.001	4	0	15	-1.307e-5	1	NC	1	NC	1
189		19	max	0	1	0	1	0	1	-4.563e-7	15	NC	1	NC	1
190			min	0	1	0	1	0	1	-7.791e-6	1	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	1.782e-6	1	NC	1	NC	1
192			min	0	1	0	1	0	1	1.048e-7	15	NC	1	NC	1
193		2	max	0	3	0	15	0	15	1.148e-5	1	NC	1	NC	1
194			min	0	2	-.002	4	0	1	6.559e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	15	2.117e-5	1	NC	1	NC	1
196			min	0	2	-.004	4	0	1	1.207e-6	15	NC	1	NC	1
197		4	max	.001	3	-.001	15	0	15	3.086e-5	1	NC	1	NC	1
198			min	-.001	2	-.006	4	0	1	1.758e-6	15	NC	1	NC	1
199		5	max	.002	3	-.002	15	0	10	4.056e-5	1	NC	1	NC	1
200			min	-.002	2	-.008	4	0	1	2.309e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	10	5.025e-5	1	NC	1	NC	1
202			min	-.002	2	-.01	4	0	1	2.861e-6	15	9257.483	4	NC	1
203		7	max	.003	3	-.003	15	0	1	5.995e-5	1	NC	1	NC	1
204			min	-.002	2	-.011	4	0	3	3.412e-6	15	8005.188	4	NC	1
205		8	max	.003	3	-.003	15	0	1	6.964e-5	1	NC	2	NC	1
206			min	-.003	2	-.013	4	0	3	3.963e-6	15	7234.508	4	NC	1
207		9	max	.004	3	-.003	15	0	1	7.933e-5	1	NC	5	NC	1
208			min	-.003	2	-.014	4	0	12	4.514e-6	15	6784.835	4	NC	1
209		10	max	.004	3	-.003	15	0	1	8.903e-5	1	NC	5	NC	1
210			min	-.004	2	-.014	4	0	15	5.065e-6	15	6578.843	4	NC	1
211		11	max	.005	3	-.003	15	0	1	9.872e-5	1	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.004	2	-.014	4	0	15	5.616e-6	15	6586.45	4	NC	1
213		max	.005	3	-.003	15	0	1	1.084e-4	1	NC	5	NC	1
214		min	-.005	2	-.014	4	0	15	6.167e-6	15	6813.281	4	NC	1
215		max	.005	3	-.003	15	.001	1	1.181e-4	1	NC	2	NC	1
216		min	-.005	2	-.013	4	0	15	6.719e-6	15	7304.322	4	NC	1
217		max	.006	3	-.003	15	.001	1	1.278e-4	1	NC	1	NC	1
218		min	-.005	2	-.011	4	0	15	7.27e-6	15	8166.69	4	NC	1
219		max	.006	3	-.002	15	.002	1	1.375e-4	1	NC	1	NC	1
220		min	-.006	2	-.01	4	0	15	7.821e-6	15	9635.729	4	NC	1
221		max	.007	3	-.002	15	.002	1	1.472e-4	1	NC	1	NC	1
222		min	-.006	2	-.008	4	0	15	8.372e-6	15	NC	1	NC	1
223		max	.007	3	-.001	15	.003	1	1.569e-4	1	NC	1	NC	1
224		min	-.007	2	-.006	3	0	15	8.923e-6	15	NC	1	NC	1
225		max	.008	3	0	15	.003	1	1.666e-4	1	NC	1	NC	1
226		min	-.007	2	-.004	3	0	15	9.474e-6	15	NC	1	NC	1
227		max	.008	3	0	2	.004	1	1.763e-4	1	NC	1	NC	1
228		min	-.007	2	-.003	3	0	15	1.003e-5	15	NC	1	NC	1
229	M4	max	.002	1	.007	2	0	15	7.021e-5	1	NC	1	NC	2
230		min	0	15	-.009	3	-.004	1	4.017e-6	15	NC	1	6719.427	1
231		max	.002	1	.007	2	0	15	7.021e-5	1	NC	1	NC	2
232		min	0	15	-.008	3	-.003	1	4.017e-6	15	NC	1	7280.522	1
233		max	.002	1	.006	2	0	15	7.021e-5	1	NC	1	NC	2
234		min	0	15	-.008	3	-.003	1	4.017e-6	15	NC	1	7950.041	1
235		max	.001	1	.006	2	0	15	7.021e-5	1	NC	1	NC	2
236		min	0	15	-.007	3	-.003	1	4.017e-6	15	NC	1	8755.85	1
237		max	.001	1	.006	2	0	15	7.021e-5	1	NC	1	NC	2
238		min	0	15	-.007	3	-.003	1	4.017e-6	15	NC	1	9735.839	1
239		max	.001	1	.005	2	0	15	7.021e-5	1	NC	1	NC	1
240		min	0	15	-.006	3	-.002	1	4.017e-6	15	NC	1	NC	1
241		max	.001	1	.005	2	0	15	7.021e-5	1	NC	1	NC	1
242		min	0	15	-.006	3	-.002	1	4.017e-6	15	NC	1	NC	1
243		max	.001	1	.004	2	0	15	7.021e-5	1	NC	1	NC	1
244		min	0	15	-.005	3	-.002	1	4.017e-6	15	NC	1	NC	1
245		max	0	1	.004	2	0	15	7.021e-5	1	NC	1	NC	1
246		min	0	15	-.005	3	-.001	1	4.017e-6	15	NC	1	NC	1
247		max	0	1	.004	2	0	15	7.021e-5	1	NC	1	NC	1
248		min	0	15	-.004	3	-.001	1	4.017e-6	15	NC	1	NC	1
249		max	0	1	.003	2	0	15	7.021e-5	1	NC	1	NC	1
250		min	0	15	-.004	3	-.001	1	4.017e-6	15	NC	1	NC	1
251		max	0	1	.003	2	0	15	7.021e-5	1	NC	1	NC	1
252		min	0	15	-.003	3	0	1	4.017e-6	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	7.021e-5	1	NC	1	NC	1
254		min	0	15	-.003	3	0	1	4.017e-6	15	NC	1	NC	1
255		max	0	1	.002	2	0	15	7.021e-5	1	NC	1	NC	1
256		min	0	15	-.002	3	0	1	4.017e-6	15	NC	1	NC	1
257		max	0	1	.002	2	0	15	7.021e-5	1	NC	1	NC	1
258		min	0	15	-.002	3	0	1	4.017e-6	15	NC	1	NC	1
259		max	0	1	.001	2	0	15	7.021e-5	1	NC	1	NC	1
260		min	0	15	-.001	3	0	1	4.017e-6	15	NC	1	NC	1
261		max	0	1	0	2	0	15	7.021e-5	1	NC	1	NC	1
262		min	0	15	0	3	0	1	4.017e-6	15	NC	1	NC	1
263		max	0	1	0	2	0	15	7.021e-5	1	NC	1	NC	1
264		min	0	15	0	3	0	1	4.017e-6	15	NC	1	NC	1
265		max	0	1	0	1	0	1	7.021e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	4.017e-6	15	NC	1	NC	1
267	M6	max	.022	2	.034	2	0	1	0	1	NC	4	NC	1
268		min	-.032	3	-.048	3	0	1	0	1	1599.027	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.02	2	.031	2	0	1	0	1	NC	4	NC	1
270		min	-.03	3	-.045	3	0	1	0	1	1692.785	3	NC	1
271	3	max	.019	2	.028	2	0	1	0	1	NC	4	NC	1
272		min	-.029	3	-.043	3	0	1	0	1	1798.361	3	NC	1
273	4	max	.018	2	.025	2	0	1	0	1	NC	4	NC	1
274		min	-.027	3	-.04	3	0	1	0	1	1918.265	3	NC	1
275	5	max	.017	2	.022	2	0	1	0	1	NC	4	NC	1
276		min	-.025	3	-.037	3	0	1	0	1	2055.718	3	NC	1
277	6	max	.016	2	.019	2	0	1	0	1	NC	4	NC	1
278		min	-.023	3	-.035	3	0	1	0	1	2214.94	3	NC	1
279	7	max	.014	2	.017	2	0	1	0	1	NC	1	NC	1
280		min	-.021	3	-.032	3	0	1	0	1	2401.554	3	NC	1
281	8	max	.013	2	.014	2	0	1	0	1	NC	1	NC	1
282		min	-.02	3	-.029	3	0	1	0	1	2623.234	3	NC	1
283	9	max	.012	2	.012	2	0	1	0	1	NC	1	NC	1
284		min	-.018	3	-.027	3	0	1	0	1	2890.725	3	NC	1
285	10	max	.011	2	.01	2	0	1	0	1	NC	1	NC	1
286		min	-.016	3	-.024	3	0	1	0	1	3219.562	3	NC	1
287	11	max	.01	2	.008	2	0	1	0	1	NC	1	NC	1
288		min	-.014	3	-.021	3	0	1	0	1	3633.052	3	NC	1
289	12	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
290		min	-.012	3	-.018	3	0	1	0	1	4167.846	3	NC	1
291	13	max	.007	2	.004	2	0	1	0	1	NC	1	NC	1
292		min	-.011	3	-.016	3	0	1	0	1	4885.05	3	NC	1
293	14	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.009	3	-.013	3	0	1	0	1	5894.703	3	NC	1
295	15	max	.005	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.007	3	-.01	3	0	1	0	1	7416.943	3	NC	1
297	16	max	.004	2	0	2	0	1	0	1	NC	1	NC	1
298		min	-.005	3	-.008	3	0	1	0	1	9965.532	3	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.002	3	-.003	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.003	3	0	2	0	1	0	1	NC	1	NC	1
310		min	-.003	2	-.006	3	0	1	0	1	NC	1	NC	1
311	4	max	.004	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.004	2	-.008	3	0	1	0	1	NC	1	NC	1
313	5	max	.006	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.005	2	-.01	3	0	1	0	1	NC	1	NC	1
315	6	max	.007	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.007	2	-.012	3	0	1	0	1	8819.79	3	NC	1
317	7	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
318		min	-.008	2	-.014	3	0	1	0	1	7881.887	3	NC	1
319	8	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.009	2	-.015	3	0	1	0	1	7308.054	4	NC	1
321	9	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.011	2	-.016	3	0	1	0	1	6849.687	4	NC	1
323	10	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.012	2	-.017	3	0	1	0	1	6638.343	4	NC	1
325	11	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.013	2	-.017	3	0	1	0	1	6643.145	4	NC	1
327		12	max	.015	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.015	2	-.017	3	0	1	0	1	6869.407	4	NC	1
329		13	max	.017	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.016	2	-.016	3	0	1	0	1	7362.21	4	NC	1
331		14	max	.018	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.018	2	-.015	3	0	1	0	1	8229.281	4	NC	1
333		15	max	.019	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.019	2	-.014	3	0	1	0	1	9707.519	4	NC	1
335		16	max	.021	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.02	2	-.013	3	0	1	0	1	NC	1	NC	1
337		17	max	.022	3	0	2	0	1	0	1	NC	1	NC	1
338			min	-.022	2	-.011	3	0	1	0	1	NC	1	NC	1
339		18	max	.023	3	0	2	0	1	0	1	NC	1	NC	1
340			min	-.023	2	-.009	3	0	1	0	1	NC	1	NC	1
341		19	max	.025	3	.002	2	0	1	0	1	NC	1	NC	1
342			min	-.024	2	-.008	3	0	1	0	1	NC	1	NC	1
343	M8	1	max	.004	2	.024	2	0	1	0	1	NC	1	NC	1
344			min	0	15	-.026	3	0	1	0	1	NC	1	NC	1
345		2	max	.004	2	.023	2	0	1	0	1	NC	1	NC	1
346			min	0	15	-.024	3	0	1	0	1	NC	1	NC	1
347		3	max	.004	2	.021	2	0	1	0	1	NC	1	NC	1
348			min	0	15	-.023	3	0	1	0	1	NC	1	NC	1
349		4	max	.004	2	.02	2	0	1	0	1	NC	1	NC	1
350			min	0	15	-.022	3	0	1	0	1	NC	1	NC	1
351		5	max	.003	2	.019	2	0	1	0	1	NC	1	NC	1
352			min	0	15	-.02	3	0	1	0	1	NC	1	NC	1
353		6	max	.003	2	.017	2	0	1	0	1	NC	1	NC	1
354			min	0	15	-.019	3	0	1	0	1	NC	1	NC	1
355		7	max	.003	2	.016	2	0	1	0	1	NC	1	NC	1
356			min	0	15	-.017	3	0	1	0	1	NC	1	NC	1
357		8	max	.003	2	.015	2	0	1	0	1	NC	1	NC	1
358			min	0	15	-.016	3	0	1	0	1	NC	1	NC	1
359		9	max	.002	2	.013	2	0	1	0	1	NC	1	NC	1
360			min	0	15	-.014	3	0	1	0	1	NC	1	NC	1
361		10	max	.002	2	.012	2	0	1	0	1	NC	1	NC	1
362			min	0	15	-.013	3	0	1	0	1	NC	1	NC	1
363		11	max	.002	2	.011	2	0	1	0	1	NC	1	NC	1
364			min	0	15	-.012	3	0	1	0	1	NC	1	NC	1
365		12	max	.002	2	.009	2	0	1	0	1	NC	1	NC	1
366			min	0	15	-.01	3	0	1	0	1	NC	1	NC	1
367		13	max	.001	2	.008	2	0	1	0	1	NC	1	NC	1
368			min	0	15	-.009	3	0	1	0	1	NC	1	NC	1
369		14	max	.001	2	.007	2	0	1	0	1	NC	1	NC	1
370			min	0	15	-.007	3	0	1	0	1	NC	1	NC	1
371		15	max	0	2	.005	2	0	1	0	1	NC	1	NC	1
372			min	0	15	-.006	3	0	1	0	1	NC	1	NC	1
373		16	max	0	2	.004	2	0	1	0	1	NC	1	NC	1
374			min	0	15	-.004	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.003	2	0	1	0	1	NC	1	NC	1
376			min	0	15	-.003	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	.001	2	0	1	0	1	NC	1	NC	1
378			min	0	15	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.007	2	.01	2	0	15	1.029e-4	1	NC	1	NC	1
382			min	-.01	3	-.016	3	-.004	1	5.942e-6	15	7453.502	2	NC	1





Company : Schletter, Inc.  
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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
383	2	max	.007	2	.009	2	0	15	9.758e-5	1	NC	1	NC	1
384		min	-.01	3	-.015	3	-.004	1	5.637e-6	15	8642.116	2	NC	1
385	3	max	.006	2	.008	2	0	15	9.23e-5	1	NC	1	NC	1
386		min	-.009	3	-.015	3	-.003	1	5.332e-6	15	NC	1	NC	1
387	4	max	.006	2	.006	2	0	15	8.702e-5	1	NC	1	NC	1
388		min	-.009	3	-.014	3	-.003	1	5.028e-6	15	NC	1	NC	1
389	5	max	.005	2	.005	2	0	15	8.174e-5	1	NC	1	NC	1
390		min	-.008	3	-.014	3	-.003	1	4.723e-6	15	NC	1	NC	1
391	6	max	.005	2	.004	2	0	15	7.646e-5	1	NC	1	NC	1
392		min	-.007	3	-.013	3	-.002	1	4.418e-6	15	NC	1	NC	1
393	7	max	.005	2	.002	2	0	15	7.117e-5	1	NC	1	NC	1
394		min	-.007	3	-.012	3	-.002	1	4.113e-6	15	NC	1	NC	1
395	8	max	.004	2	.001	2	0	15	6.589e-5	1	NC	1	NC	1
396		min	-.006	3	-.012	3	-.002	1	3.809e-6	15	NC	1	NC	1
397	9	max	.004	2	0	2	0	15	6.061e-5	1	NC	1	NC	1
398		min	-.006	3	-.011	3	-.001	1	3.504e-6	15	NC	1	NC	1
399	10	max	.004	2	0	2	0	15	5.533e-5	1	NC	1	NC	1
400		min	-.005	3	-.01	3	-.001	1	3.199e-6	15	NC	1	NC	1
401	11	max	.003	2	0	2	0	15	5.005e-5	1	NC	1	NC	1
402		min	-.005	3	-.009	3	0	1	2.894e-6	15	NC	1	NC	1
403	12	max	.003	2	-.001	15	0	15	4.476e-5	1	NC	1	NC	1
404		min	-.004	3	-.008	3	0	1	2.59e-6	15	NC	1	NC	1
405	13	max	.002	2	-.001	15	0	15	3.948e-5	1	NC	1	NC	1
406		min	-.003	3	-.007	3	0	1	2.285e-6	15	NC	1	NC	1
407	14	max	.002	2	-.001	15	0	15	3.42e-5	1	NC	1	NC	1
408		min	-.003	3	-.006	3	0	1	1.98e-6	15	NC	1	NC	1
409	15	max	.002	2	-.001	15	0	15	2.892e-5	1	NC	1	NC	1
410		min	-.002	3	-.005	3	0	1	1.675e-6	15	NC	1	NC	1
411	16	max	.001	2	0	15	0	15	2.364e-5	1	NC	1	NC	1
412		min	-.002	3	-.004	3	0	1	1.371e-6	15	NC	1	NC	1
413	17	max	0	2	0	15	0	15	1.835e-5	1	NC	1	NC	1
414		min	-.001	3	-.003	3	0	1	1.066e-6	15	NC	1	NC	1
415	18	max	0	2	0	15	0	15	1.307e-5	1	NC	1	NC	1
416		min	0	3	-.001	4	0	1	7.61e-7	15	NC	1	NC	1
417	19	max	0	1	0	1	0	1	7.791e-6	1	NC	1	NC	1
418		min	0	1	0	1	0	1	4.563e-7	15	NC	1	NC	1
419	M11	1	max	0	1	0	1	1	-1.048e-7	15	NC	1	NC	1
420		min	0	1	0	1	0	1	-1.782e-6	1	NC	1	NC	1
421	2	max	0	3	0	15	0	1	-6.559e-7	15	NC	1	NC	1
422		min	0	2	-.002	4	0	15	-1.148e-5	1	NC	1	NC	1
423	3	max	0	3	0	15	0	1	-1.207e-6	15	NC	1	NC	1
424		min	0	2	-.004	4	0	15	-2.117e-5	1	NC	1	NC	1
425	4	max	.001	3	-.001	15	0	1	-1.758e-6	15	NC	1	NC	1
426		min	-.001	2	-.006	4	0	15	-3.086e-5	1	NC	1	NC	1
427	5	max	.002	3	-.002	15	0	1	-2.309e-6	15	NC	1	NC	1
428		min	-.002	2	-.008	4	0	10	-4.056e-5	1	NC	1	NC	1
429	6	max	.002	3	-.002	15	0	1	-2.861e-6	15	NC	1	NC	1
430		min	-.002	2	-.01	4	0	10	-5.025e-5	1	9257.483	4	NC	1
431	7	max	.003	3	-.003	15	0	3	-3.412e-6	15	NC	1	NC	1
432		min	-.002	2	-.011	4	0	1	-5.995e-5	1	8005.188	4	NC	1
433	8	max	.003	3	-.003	15	0	3	-3.963e-6	15	NC	2	NC	1
434		min	-.003	2	-.013	4	0	1	-6.964e-5	1	7234.508	4	NC	1
435	9	max	.004	3	-.003	15	0	12	-4.514e-6	15	NC	5	NC	1
436		min	-.003	2	-.014	4	0	1	-7.933e-5	1	6784.835	4	NC	1
437	10	max	.004	3	-.003	15	0	15	-5.065e-6	15	NC	5	NC	1
438		min	-.004	2	-.014	4	0	1	-8.903e-5	1	6578.843	4	NC	1
439	11	max	.005	3	-.003	15	0	15	-5.616e-6	15	NC	5	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440			min	-.004	2	-.014	4	0	1	-9.872e-5	1	6586.45	4	NC	1
441		12	max	.005	3	-.003	15	0	15	-6.167e-6	15	NC	5	NC	1
442			min	-.005	2	-.014	4	0	1	-1.084e-4	1	6813.281	4	NC	1
443		13	max	.005	3	-.003	15	0	15	-6.719e-6	15	NC	2	NC	1
444			min	-.005	2	-.013	4	-.001	1	-1.181e-4	1	7304.322	4	NC	1
445		14	max	.006	3	-.003	15	0	15	-7.27e-6	15	NC	1	NC	1
446			min	-.005	2	-.011	4	-.001	1	-1.278e-4	1	8166.69	4	NC	1
447		15	max	.006	3	-.002	15	0	15	-7.821e-6	15	NC	1	NC	1
448			min	-.006	2	-.01	4	-.002	1	-1.375e-4	1	9635.729	4	NC	1
449		16	max	.007	3	-.002	15	0	15	-8.372e-6	15	NC	1	NC	1
450			min	-.006	2	-.008	4	-.002	1	-1.472e-4	1	NC	1	NC	1
451		17	max	.007	3	-.001	15	0	15	-8.923e-6	15	NC	1	NC	1
452			min	-.007	2	-.006	3	-.003	1	-1.569e-4	1	NC	1	NC	1
453		18	max	.008	3	0	15	0	15	-9.474e-6	15	NC	1	NC	1
454			min	-.007	2	-.004	3	-.003	1	-1.666e-4	1	NC	1	NC	1
455		19	max	.008	3	0	2	0	15	-1.003e-5	15	NC	1	NC	1
456			min	-.007	2	-.003	3	-.004	1	-1.763e-4	1	NC	1	NC	1
457	M12	1	max	.002	1	.007	2	.004	1	-4.017e-6	15	NC	1	NC	2
458			min	0	15	-.009	3	0	15	-7.021e-5	1	NC	1	6719.427	1
459		2	max	.002	1	.007	2	.003	1	-4.017e-6	15	NC	1	NC	2
460			min	0	15	-.008	3	0	15	-7.021e-5	1	NC	1	7280.522	1
461		3	max	.002	1	.006	2	.003	1	-4.017e-6	15	NC	1	NC	2
462			min	0	15	-.008	3	0	15	-7.021e-5	1	NC	1	7950.041	1
463		4	max	.001	1	.006	2	.003	1	-4.017e-6	15	NC	1	NC	2
464			min	0	15	-.007	3	0	15	-7.021e-5	1	NC	1	8755.85	1
465		5	max	.001	1	.006	2	.003	1	-4.017e-6	15	NC	1	NC	2
466			min	0	15	-.007	3	0	15	-7.021e-5	1	NC	1	9735.839	1
467		6	max	.001	1	.005	2	.002	1	-4.017e-6	15	NC	1	NC	1
468			min	0	15	-.006	3	0	15	-7.021e-5	1	NC	1	NC	1
469		7	max	.001	1	.005	2	.002	1	-4.017e-6	15	NC	1	NC	1
470			min	0	15	-.006	3	0	15	-7.021e-5	1	NC	1	NC	1
471		8	max	.001	1	.004	2	.002	1	-4.017e-6	15	NC	1	NC	1
472			min	0	15	-.005	3	0	15	-7.021e-5	1	NC	1	NC	1
473		9	max	0	1	.004	2	.001	1	-4.017e-6	15	NC	1	NC	1
474			min	0	15	-.005	3	0	15	-7.021e-5	1	NC	1	NC	1
475		10	max	0	1	.004	2	.001	1	-4.017e-6	15	NC	1	NC	1
476			min	0	15	-.004	3	0	15	-7.021e-5	1	NC	1	NC	1
477		11	max	0	1	.003	2	.001	1	-4.017e-6	15	NC	1	NC	1
478			min	0	15	-.004	3	0	15	-7.021e-5	1	NC	1	NC	1
479		12	max	0	1	.003	2	0	1	-4.017e-6	15	NC	1	NC	1
480			min	0	15	-.003	3	0	15	-7.021e-5	1	NC	1	NC	1
481		13	max	0	1	.002	2	0	1	-4.017e-6	15	NC	1	NC	1
482			min	0	15	-.003	3	0	15	-7.021e-5	1	NC	1	NC	1
483		14	max	0	1	.002	2	0	1	-4.017e-6	15	NC	1	NC	1
484			min	0	15	-.002	3	0	15	-7.021e-5	1	NC	1	NC	1
485		15	max	0	1	.002	2	0	1	-4.017e-6	15	NC	1	NC	1
486			min	0	15	-.002	3	0	15	-7.021e-5	1	NC	1	NC	1
487		16	max	0	1	.001	2	0	1	-4.017e-6	15	NC	1	NC	1
488			min	0	15	-.001	3	0	15	-7.021e-5	1	NC	1	NC	1
489		17	max	0	1	0	2	0	1	-4.017e-6	15	NC	1	NC	1
490			min	0	15	0	3	0	15	-7.021e-5	1	NC	1	NC	1
491		18	max	0	1	0	2	0	1	-4.017e-6	15	NC	1	NC	1
492			min	0	15	0	3	0	15	-7.021e-5	1	NC	1	NC	1
493		19	max	0	1	0	1	0	1	-4.017e-6	15	NC	1	NC	1
494			min	0	1	0	1	0	1	-7.021e-5	1	NC	1	NC	1
495	M1	1	max	.01	3	.112	2	0	1	5.338e-3	2	NC	1	NC	1
496			min	-.006	2	-.031	3	0	15	-1.324e-2	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497	2	max	.01	3	.052	2	0	15	2.618e-3	2	NC	4	NC	1
498		min	-.006	2	-.011	3	-.003	1	-6.554e-3	3	1909.374	2	NC	1
499	3	max	.01	3	.017	3	0	15	2.566e-5	10	NC	5	NC	1
500		min	-.006	2	-.012	2	-.004	1	-9.236e-5	3	926.228	2	NC	1
501	4	max	.01	3	.058	3	0	15	2.791e-3	2	NC	5	NC	1
502		min	-.006	2	-.083	2	-.004	1	-3.15e-3	3	590.258	2	NC	1
503	5	max	.01	3	.108	3	0	15	5.559e-3	2	NC	5	NC	1
504		min	-.006	2	-.156	2	-.002	1	-6.208e-3	3	429.476	2	NC	1
505	6	max	.009	3	.161	3	0	15	8.327e-3	2	NC	5	NC	1
506		min	-.006	2	-.226	2	-.001	1	-9.266e-3	3	340.404	2	NC	1
507	7	max	.009	3	.211	3	0	1	1.109e-2	2	NC	15	NC	1
508		min	-.006	2	-.289	2	0	3	-1.232e-2	3	287.572	2	NC	1
509	8	max	.009	3	.252	3	0	1	1.386e-2	2	NC	15	NC	1
510		min	-.006	2	-.338	2	0	15	-1.538e-2	3	256.21	2	NC	1
511	9	max	.009	3	.279	3	0	15	1.569e-2	2	NC	15	NC	1
512		min	-.006	2	-.369	2	0	1	-1.581e-2	3	239.839	2	NC	1
513	10	max	.009	3	.288	3	0	1	1.688e-2	2	NC	15	NC	1
514		min	-.006	2	-.379	2	0	15	-1.448e-2	3	235.071	2	NC	1
515	11	max	.008	3	.281	3	0	1	1.807e-2	2	NC	15	NC	1
516		min	-.006	2	-.368	2	0	15	-1.316e-2	3	240.805	2	NC	1
517	12	max	.008	3	.257	3	0	15	1.742e-2	2	NC	15	NC	1
518		min	-.005	2	-.336	2	0	1	-1.145e-2	3	259.09	2	NC	1
519	13	max	.008	3	.219	3	0	15	1.396e-2	2	NC	15	NC	1
520		min	-.005	2	-.284	2	0	1	-9.167e-3	3	294.457	2	NC	1
521	14	max	.008	3	.171	3	0	1	1.051e-2	2	NC	5	NC	1
522		min	-.005	2	-.218	2	0	15	-6.88e-3	3	354.897	2	NC	1
523	15	max	.008	3	.117	3	.002	1	7.062e-3	2	NC	5	NC	1
524		min	-.005	2	-.146	2	0	15	-4.593e-3	3	458.865	2	NC	1
525	16	max	.007	3	.061	3	.003	1	3.611e-3	2	NC	5	NC	1
526		min	-.005	2	-.074	2	0	15	-2.307e-3	3	651.208	2	NC	1
527	17	max	.007	3	.006	3	.004	1	2.619e-4	1	NC	5	NC	1
528		min	-.005	2	-.007	2	0	15	-1.973e-5	3	1061.986	2	NC	1
529	18	max	.007	3	.049	2	.003	1	4.809e-3	2	NC	4	NC	1
530		min	-.005	2	-.043	3	0	15	-1.948e-3	3	2251.118	2	NC	1
531	19	max	.007	3	.099	2	0	15	9.647e-3	2	NC	1	NC	1
532		min	-.005	2	-.09	3	0	1	-3.974e-3	3	NC	1	NC	1
533	M5	1	max	.03	.197	2	0	1	0	1	NC	1	NC	1
534		min	-.022	2	-.003	3	0	1	0	1	NC	1	NC	1
535	2	max	.03	3	.087	2	0	1	0	1	NC	4	NC	1
536		min	-.022	2	.002	15	0	1	0	1	1055.709	2	NC	1
537	3	max	.03	3	.051	3	0	1	0	1	NC	5	NC	1
538		min	-.022	2	-.037	2	0	1	0	1	495.631	2	NC	1
539	4	max	.03	3	.135	3	0	1	0	1	NC	5	NC	1
540		min	-.021	2	-.186	2	0	1	0	1	302.509	2	NC	1
541	5	max	.029	3	.25	3	0	1	0	1	NC	15	NC	1
542		min	-.021	2	-.348	2	0	1	0	1	212.444	2	NC	1
543	6	max	.028	3	.38	3	0	1	0	1	8926.95	15	NC	1
544		min	-.021	2	-.509	2	0	1	0	1	163.941	2	NC	1
545	7	max	.028	3	.505	3	0	1	0	1	7369.716	15	NC	1
546		min	-.02	2	-.655	2	0	1	0	1	135.84	2	NC	1
547	8	max	.027	3	.611	3	0	1	0	1	6466.866	15	NC	1
548		min	-.02	2	-.772	2	0	1	0	1	119.463	2	NC	1
549	9	max	.026	3	.678	3	0	1	0	1	6004.862	15	NC	1
550		min	-.02	2	-.847	2	0	1	0	1	111.05	2	NC	1
551	10	max	.026	3	.701	3	0	1	0	1	5865.906	15	NC	1
552		min	-.019	2	-.872	2	0	1	0	1	108.606	2	NC	1
553	11	max	.025	3	.682	3	0	1	0	1	6005.449	15	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Nov 18, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554		min	-.019	2	-.846	2	0	1	0	1	111.545	2	NC	1
555	12	max	.025	3	.623	3	0	1	0	1	6468.209	15	NC	1
556		min	-.019	2	-.768	2	0	1	0	1	121.116	2	NC	1
557	13	max	.024	3	.527	3	0	1	0	1	7372.317	15	NC	1
558		min	-.018	2	-.642	2	0	1	0	1	140.196	2	NC	1
559	14	max	.023	3	.407	3	0	1	0	1	8931.828	15	NC	1
560		min	-.018	2	-.487	2	0	1	0	1	173.905	2	NC	1
561	15	max	.023	3	.274	3	0	1	0	1	NC	15	NC	1
562		min	-.018	2	-.319	2	0	1	0	1	234.541	2	NC	1
563	16	max	.022	3	.141	3	0	1	0	1	NC	5	NC	1
564		min	-.017	2	-.158	2	0	1	0	1	353.599	2	NC	1
565	17	max	.021	3	.017	3	0	1	0	1	NC	5	NC	1
566		min	-.017	2	-.02	2	0	1	0	1	625.428	2	NC	1
567	18	max	.021	3	.079	2	0	1	0	1	NC	4	NC	1
568		min	-.017	2	-.086	3	0	1	0	1	1388.956	3	NC	1
569	19	max	.021	3	.158	2	0	1	0	1	NC	1	NC	1
570		min	-.017	2	-.178	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.01	.112	2	0	15	1.324e-2	3	NC	1	NC	1
572		min	-.006	2	-.031	3	0	1	-5.338e-3	2	NC	1	NC	1
573	2	max	.01	3	.052	2	.003	1	6.554e-3	3	NC	4	NC	1
574		min	-.006	2	-.011	3	0	15	-2.618e-3	2	1909.374	2	NC	1
575	3	max	.01	3	.017	3	.004	1	9.236e-5	3	NC	5	NC	1
576		min	-.006	2	-.012	2	0	15	-2.566e-5	10	926.228	2	NC	1
577	4	max	.01	3	.058	3	.004	1	3.15e-3	3	NC	5	NC	1
578		min	-.006	2	-.083	2	0	15	-2.791e-3	2	590.258	2	NC	1
579	5	max	.01	3	.108	3	.002	1	6.208e-3	3	NC	5	NC	1
580		min	-.006	2	-.156	2	0	15	-5.559e-3	2	429.476	2	NC	1
581	6	max	.009	3	.161	3	.001	1	9.266e-3	3	NC	5	NC	1
582		min	-.006	2	-.226	2	0	15	-8.327e-3	2	340.404	2	NC	1
583	7	max	.009	3	.211	3	0	3	1.232e-2	3	NC	15	NC	1
584		min	-.006	2	-.289	2	0	1	-1.109e-2	2	287.572	2	NC	1
585	8	max	.009	3	.252	3	0	15	1.538e-2	3	NC	15	NC	1
586		min	-.006	2	-.338	2	0	1	-1.386e-2	2	256.21	2	NC	1
587	9	max	.009	3	.279	3	0	1	1.581e-2	3	NC	15	NC	1
588		min	-.006	2	-.369	2	0	15	-1.569e-2	2	239.839	2	NC	1
589	10	max	.009	3	.288	3	0	15	1.448e-2	3	NC	15	NC	1
590		min	-.006	2	-.379	2	0	1	-1.688e-2	2	235.071	2	NC	1
591	11	max	.008	3	.281	3	0	15	1.316e-2	3	NC	15	NC	1
592		min	-.006	2	-.368	2	0	1	-1.807e-2	2	240.805	2	NC	1
593	12	max	.008	3	.257	3	0	1	1.145e-2	3	NC	15	NC	1
594		min	-.005	2	-.336	2	0	15	-1.742e-2	2	259.09	2	NC	1
595	13	max	.008	3	.219	3	0	1	9.167e-3	3	NC	15	NC	1
596		min	-.005	2	-.284	2	0	15	-1.396e-2	2	294.457	2	NC	1
597	14	max	.008	3	.171	3	0	15	6.88e-3	3	NC	5	NC	1
598		min	-.005	2	-.218	2	0	1	-1.051e-2	2	354.897	2	NC	1
599	15	max	.008	3	.117	3	0	15	4.593e-3	3	NC	5	NC	1
600		min	-.005	2	-.146	2	-.002	1	-7.062e-3	2	458.865	2	NC	1
601	16	max	.007	3	.061	3	0	15	2.307e-3	3	NC	5	NC	1
602		min	-.005	2	-.074	2	-.003	1	-3.611e-3	2	651.208	2	NC	1
603	17	max	.007	3	.006	3	0	15	1.973e-5	3	NC	5	NC	1
604		min	-.005	2	-.007	2	-.004	1	-2.619e-4	1	1061.986	2	NC	1
605	18	max	.007	3	.049	2	0	15	1.948e-3	3	NC	4	NC	1
606		min	-.005	2	-.043	3	-.003	1	-4.809e-3	2	2251.118	2	NC	1
607	19	max	.007	3	.099	2	0	1	3.974e-3	3	NC	1	NC	1
608		min	-.005	2	-.09	3	0	15	-9.647e-3	2	NC	1	NC	1



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Project:	Standard PVMax - Worst Case, 14-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



#### Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 1723  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

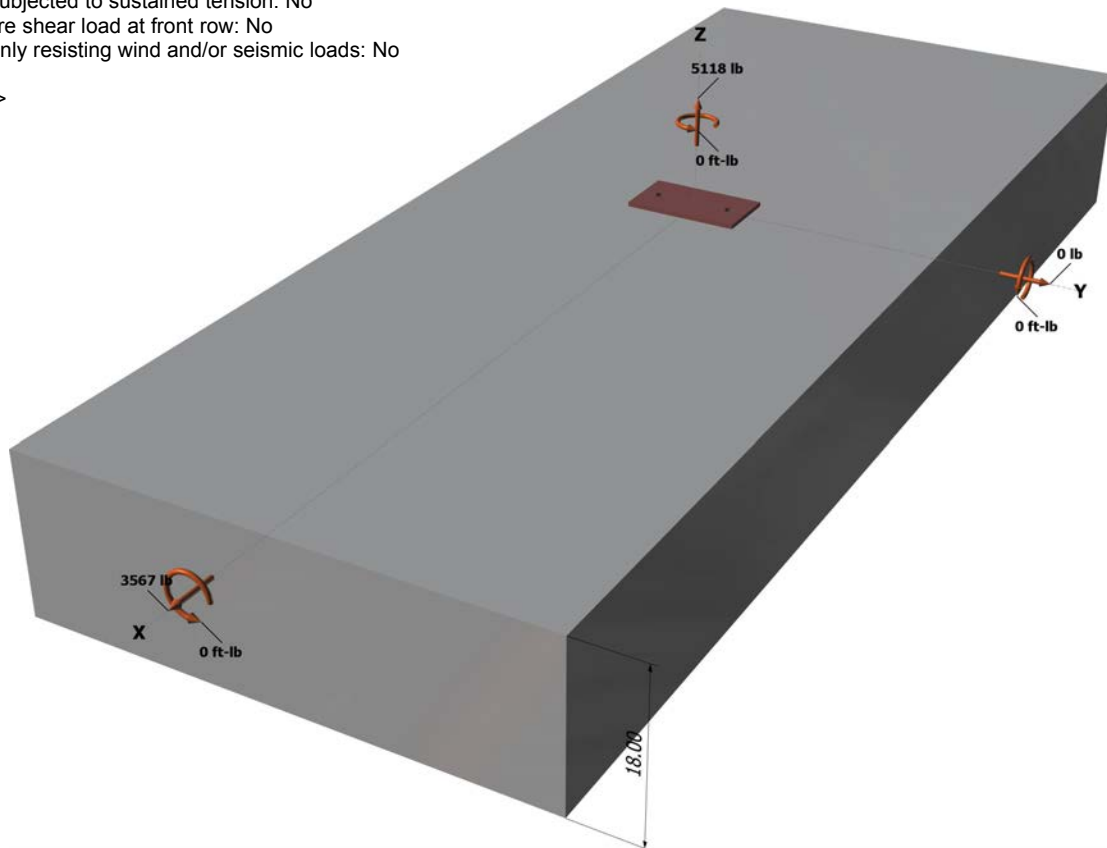
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

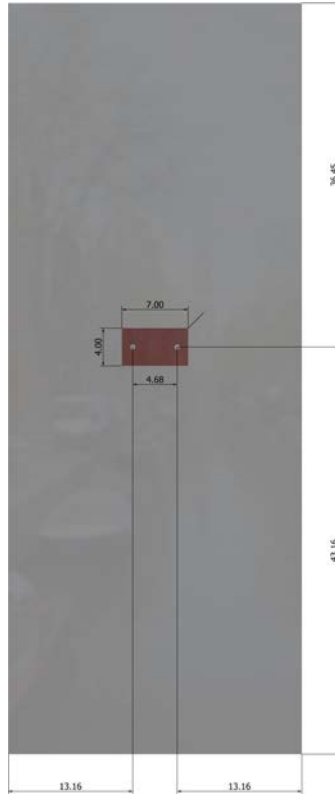
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2559.0	1783.5	0.0	1783.5
2	2559.0	1783.5	0.0	1783.5
Sum	5118.0	3567.0	0.0	3567.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5118  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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## 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

## 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \Psi_{ec,V} \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ec,V}$	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
558.00	648.00	1.000	0.919	1.000	1.000	15593	0.70	8641

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	13.16	17908

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \Psi_{ed,V} \Psi_{c,V} \Psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\Psi_{ed,V}$	$\Psi_{c,V}$	$\Psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
710.64	779.34	1.000	1.000	1.000	17908	0.70	22862

## 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \Psi_{ed,Na} \Psi_{g,Na} \Psi_{ec,Na} \Psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \Psi_{ec,N} \Psi_{ed,N} \Psi_{c,N} \Psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\Psi_{ed,Na}$	$\Psi_{g,Na}$	$\Psi_{ec,Na}$	$\Psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\Psi_{ec,N}$	$\Psi_{ed,N}$	$\Psi_{c,N}$	$\Psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2559	6071	0.42	Pass	
Concrete breakout	5118	10231	0.50	Pass	
<b>Adhesive</b>	<b>5118</b>	<b>8093</b>	<b>0.63</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1784</b>	<b>3156</b>	<b>0.57</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3567	8641	0.41	Pass	
Concrete breakout y-	1784	22862	0.08	Pass	
Pryout	3567	20601	0.17	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	11/17/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMax - Worst Case, 31-33 Inch Width		
Address:			
Phone:			
E-mail:			

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Sec. D.7.3	0.63	0.57	119.8 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.