



Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	35° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 35°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	14.43 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.64	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	130 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 26.53 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.2	(Pressure)
$C_{f+ BOTTOM}$ =	2	
$C_{f- TOP}$ =	-2.4	(Suction)
$C_{f- BOTTOM}$ =	-1.2	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

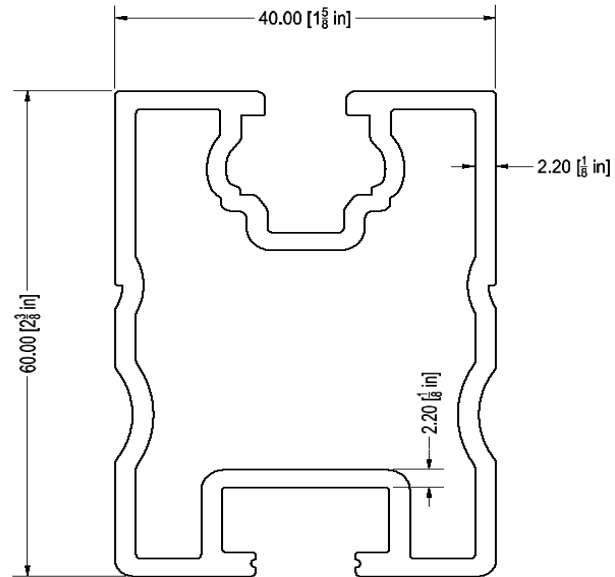
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	ProfiPlus
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	36 in
ΦF_{ty} STRONG-AXIS =	30.25 ksi
ΦF_{ty} WEAK-AXIS =	28.47 ksi
S_y =	0.51 in ³
S_x =	0.37 in ³
E =	10100 ksi
I_y =	0.60 in ⁴
I_x =	0.29 in ⁴
A =	0.90 in ²
g =	1.08 lbs/ft
M_y =	-0.255 k-ft
M_z =	-0.025 k-ft
$M_{y \text{ allowable}}$ =	1.287 k-ft
$M_{z \text{ allowable}}$ =	0.871 k-ft
Utilization =	23%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

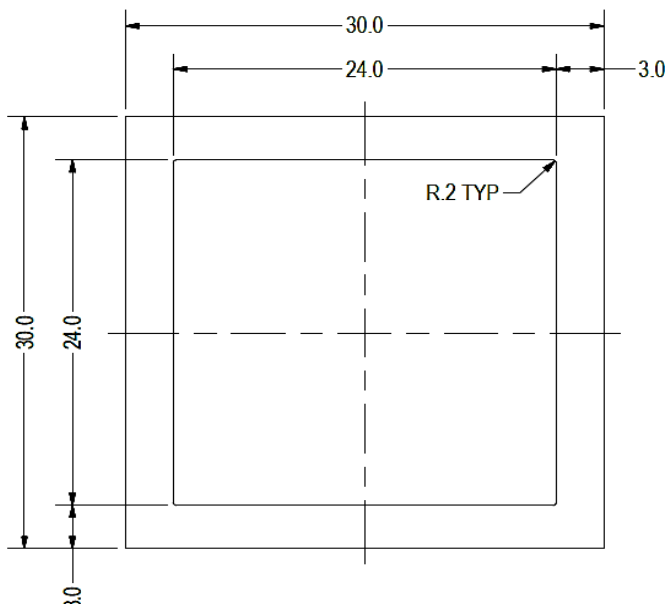
Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.66 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	-0.433 k-ft
M_z =	-0.026 k-ft
P_n =	0.295 k
$M_{y \text{ allowable}}$ =	1.455 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	37%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

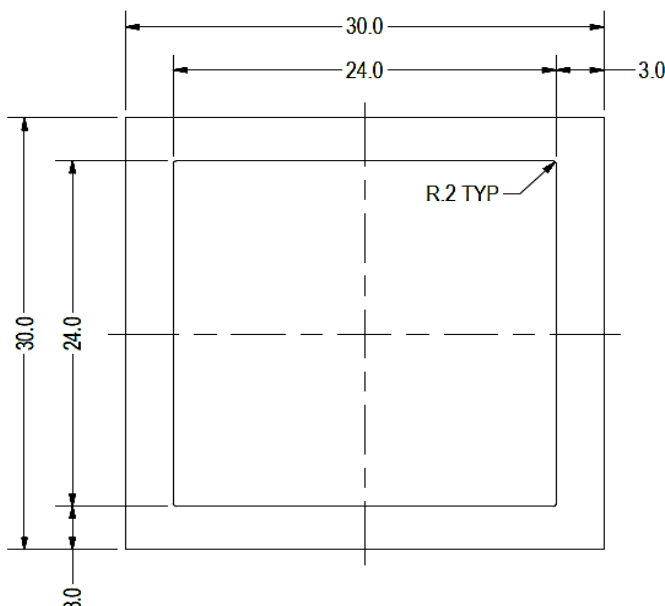
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.020 k-ft
P_n =	0.103 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	6%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

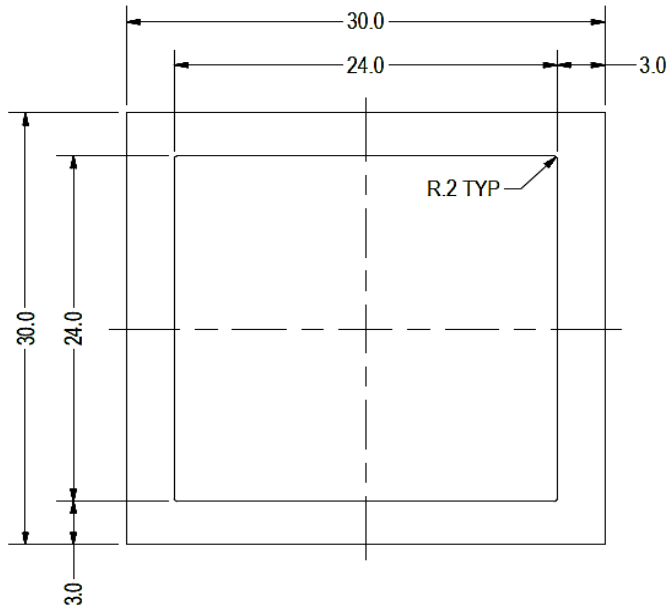
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.660 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	17%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

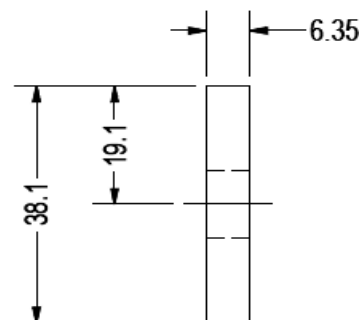
Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	42.32 in
$\Phi F_{ty \text{ AXIAL}}$ =	8.86 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.96 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.533 k
$M_{y \text{ allowable}}$ =	0.406 k-ft
$M_{z \text{ allowable}}$ =	0.406 k-ft
$P_{n \text{ allowable}}$ =	4.450 k
Utilization =	12%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.002 k-ft
P_n =	0.170 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	6%



A cross brace kit is required every 34 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

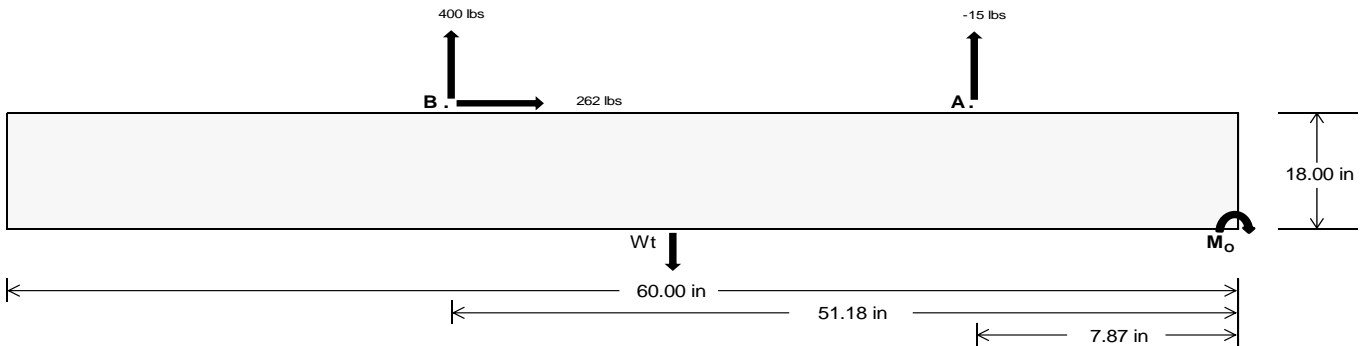
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	2.63	1665.73	k
Compressive Load =	721.74	1050.92	k
Lateral Load =	16.58	1090.99	k
Moment (Weak Axis) =	0.03	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 25081.1$ in-lbs
Resisting Force Required = 836.04 lbs
S.F. = 1.67
Weight Required = 1393.39 lbs
Minimum Width = 20 in
Weight Provided = 1812.50 lbs

Sliding

Force = 262.22 lbs
Friction = 0.4
Weight Required = 655.55 lbs
Resisting Weight = 1812.50 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 262.22 lbs
Cohesion = 130 psf
Area = 8.33 ft²
Resisting = 906.25 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 20in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 20in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

	Ballast Width			
	20 in	21 in	22 in	23 in
$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.67 \text{ ft}) =$	1813 lbs	1903 lbs	1994 lbs	2084 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in	20 in	21 in	22 in	23 in
F_A	223 lbs	223 lbs	223 lbs	223 lbs	304 lbs	304 lbs	304 lbs	304 lbs	372 lbs	372 lbs	372 lbs	372 lbs	30 lbs	30 lbs	30 lbs	30 lbs
F_B	137 lbs	137 lbs	137 lbs	137 lbs	480 lbs	480 lbs	480 lbs	480 lbs	447 lbs	447 lbs	447 lbs	447 lbs	-800 lbs	-800 lbs	-800 lbs	-800 lbs
F_V	15 lbs	15 lbs	15 lbs	15 lbs	468 lbs	468 lbs	468 lbs	468 lbs	361 lbs	361 lbs	361 lbs	361 lbs	-524 lbs	-524 lbs	-524 lbs	-524 lbs
P_{total}	2172 lbs	2263 lbs	2353 lbs	2444 lbs	2596 lbs	2687 lbs	2777 lbs	2868 lbs	2631 lbs	2722 lbs	2812 lbs	2903 lbs	317 lbs	372 lbs	426 lbs	480 lbs
M	192 lbs-ft	192 lbs-ft	192 lbs-ft	192 lbs-ft	416 lbs-ft	416 lbs-ft	416 lbs-ft	416 lbs-ft	438 lbs-ft	438 lbs-ft	438 lbs-ft	438 lbs-ft	681 lbs-ft	681 lbs-ft	681 lbs-ft	681 lbs-ft
e	0.09 ft	0.08 ft	0.08 ft	0.08 ft	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.17 ft	0.16 ft	0.16 ft	0.15 ft	2.15 ft	1.83 ft	1.60 ft	1.42 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	233.0 psf	232.2 psf	231.6 psf	231.0 psf	251.6 psf	250.0 psf	248.5 psf	247.1 psf	252.6 psf	250.9 psf	249.4 psf	248.0 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	288.3 psf	284.9 psf	281.9 psf	279.1 psf	371.5 psf	364.2 psf	357.5 psf	351.4 psf	378.8 psf	371.1 psf	364.2 psf	357.8 psf	360.0 psf	212.4 psf	172.0 psf	154.5 psf

Maximum Bearing Pressure = 379 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

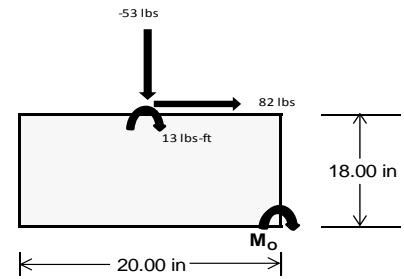
Overturning Check

$M_o = 180.1 \text{ ft-lbs}$
 Resisting Force Required = 216.11 lbs
 S.F. = 1.67
 Weight Required = 360.18 lbs
 Minimum Width = 20 in
 Weight Provided = 1812.50 lbs

A minimum 60in long x 20in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	20 in			20 in			20 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	117 lbs	15 lbs	49 lbs	172 lbs	242 lbs	120 lbs	86 lbs	-53 lbs	20 lbs
F_v	10 lbs	82 lbs	10 lbs	8 lbs	62 lbs	8 lbs	10 lbs	82 lbs	10 lbs
P_{total}	2361 lbs	2259 lbs	2293 lbs	2308 lbs	2378 lbs	2256 lbs	742 lbs	603 lbs	676 lbs
M	27 lbs-ft	136 lbs-ft	28 lbs-ft	20 lbs-ft	102 lbs-ft	22 lbs-ft	28 lbs-ft	136 lbs-ft	28 lbs-ft
e	0.01 ft	0.06 ft	0.01 ft	0.01 ft	0.04 ft	0.01 ft	0.04 ft	0.23 ft	0.04 ft
$L/6$	0.28 ft	1.55 ft	1.64 ft	1.65 ft	1.58 ft	1.65 ft	1.59 ft	1.22 ft	1.58 ft
f_{min}	271.4 sqft	212.3 sqft	263.0 sqft	268.2 sqft	241.2 sqft	261.5 sqft	77.2 sqft	13.6 sqft	69.0 sqft
f_{max}	295.2 psf	329.9 psf	287.2 psf	285.8 psf	329.6 psf	280.1 psf	101.0 psf	131.2 psf	93.2 psf



Maximum Bearing Pressure = 330 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 20in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 20in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

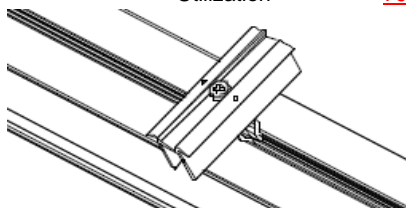
6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

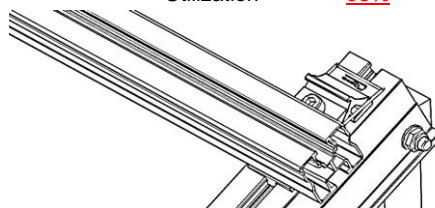
Fastening of Modules to Purlins

Maximum Uplifting Force =	0.964 k
Allowable Uplift =	1.214 k
Utilization =	<u>79%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.039 k
Allowable Uplift =	1.116 k
Utilization =	<u>93%</u>



6.2 Bolted Connections

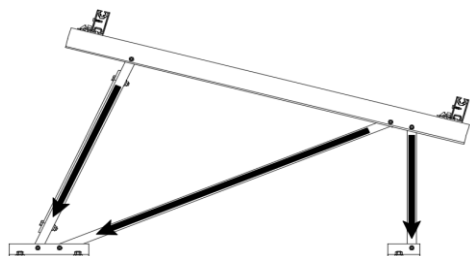
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	0.555 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>10%</u>

Diagonal Strut

Maximum Axial Load =	0.660 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>12%</u>



Rear Strut

Maximum Axial Load =	0.962 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>17%</u>

Bracing

Maximum Axial Load =	0.170 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	33.11 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.662 in
Max Drift, Δ_{MAX} =	0.04 in
	<u>0.04 ≤ 0.662. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

3.4.14

$$L_b = 36.00 \text{ in}$$

$$J = 0.255$$

$$93.7419$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.3 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 36.00 \text{ in}$$

$$J = 0.255$$

$$97.3454$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.2$$

3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.3 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.287 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &21.9891 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.25 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.455 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LSt} = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max St} = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_{LWk} = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max Wk} = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 42.32 \text{ in}$$

$$J = 0.16$$

$$111.025$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.0 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.406 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.81475 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.83406 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 8.86409 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 8.86 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 4.45 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0 \...\PVMMini 60 Cell 1V 35° 130mph 30psf 3ft 7-05.r3dPage 21



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	184.033	2	-.031	15	.035	9	0	10	0	4	0	15
30		min	-355.116	3	-.128	4	-.671	5	0	4	0	3	0	6
31	16	max	184.167	2	-.044	15	.035	9	0	10	0	4	0	15
32		min	-355.015	3	-.185	4	-.794	5	0	4	0	3	0	6
33	17	max	184.302	2	-.058	15	.035	9	0	10	0	4	0	15
34		min	-354.914	3	-.243	4	-.917	5	0	4	0	3	0	6
35	18	max	184.437	2	-.071	15	.035	9	0	10	0	9	0	15
36		min	-354.813	3	-.3	4	-1.04	5	0	4	0	3	0	6
37	19	max	184.572	2	-.085	15	.035	9	0	10	0	9	0	15
38		min	-354.711	3	-.358	4	-1.163	5	0	4	0	5	0	6
39	M3	1	max	244.621	2	1.734	.02	10	0	10	0	4	0	6
40		min	-225.109	3	.407	15	-1.301	4	0	3	0	10	0	15
41	2	max	244.551	2	1.558	.02	10	0	10	0	3	0	2	
42		min	-225.162	3	.365	15	-1.168	4	0	3	0	10	0	3
43	3	max	244.481	2	1.382	.02	10	0	10	0	3	0	2	
44		min	-225.214	3	.324	15	-1.034	4	0	3	0	5	0	3
45	4	max	244.411	2	1.205	.02	10	0	10	0	3	0	15	
46		min	-225.267	3	.283	15	-.9	4	0	3	0	5	0	4
47	5	max	244.341	2	1.029	.02	10	0	10	0	3	0	15	
48		min	-225.319	3	.241	15	-.767	4	0	3	0	5	0	4
49	6	max	244.271	2	.853	.02	10	0	10	0	3	0	15	
50		min	-225.372	3	.2	15	-.633	4	0	3	0	5	0	4
51	7	max	244.201	2	.676	.02	10	0	10	0	3	0	15	
52		min	-225.424	3	.158	15	-.499	4	0	3	0	5	0	4
53	8	max	244.131	2	.5	.02	10	0	10	0	3	0	15	
54		min	-225.477	3	.117	15	-.366	4	0	3	0	5	-.001	4
55	9	max	244.061	2	.323	.02	10	0	10	0	3	0	15	
56		min	-225.529	3	.075	15	-.232	4	0	3	0	5	-.001	4
57	10	max	243.991	2	.147	.02	10	0	10	0	3	0	15	
58		min	-225.582	3	.034	15	-.098	4	0	3	0	5	-.001	4
59	11	max	243.921	2	.007	.053	5	0	10	0	3	0	15	
60		min	-225.634	3	-.054	3	-.063	3	0	3	0	5	-.001	4
61	12	max	243.851	2	-.049	.187	5	0	10	0	3	0	15	
62		min	-225.687	3	-.206	4	-.063	3	0	3	0	5	-.001	4
63	13	max	243.781	2	-.091	.32	5	0	10	0	3	0	15	
64		min	-225.739	3	-.382	4	-.063	3	0	3	0	5	-.001	4
65	14	max	243.711	2	-.132	.454	5	0	10	0	3	0	15	
66		min	-225.792	3	-.559	4	-.063	3	0	3	0	5	-.001	4
67	15	max	243.641	2	-.174	.588	5	0	10	0	3	0	15	
68		min	-225.844	3	-.735	4	-.063	3	0	3	0	5	0	4
69	16	max	243.571	2	-.215	.721	5	0	10	0	9	0	15	
70		min	-225.897	3	-.911	4	-.063	3	0	3	0	5	0	4
71	17	max	243.501	2	-.256	.855	5	0	10	0	10	0	15	
72		min	-225.949	3	-1.088	4	-.063	3	0	3	0	4	0	4
73	18	max	243.431	2	-.298	.989	5	0	10	0	10	0	15	
74		min	-226.002	3	-1.264	4	-.063	3	0	3	0	4	0	4
75	19	max	243.361	2	-.339	1.122	5	0	10	0	5	0	1	
76		min	-226.054	3	-1.44	4	-.063	3	0	3	0	3	0	1
77	M4	1	max	190.603	1	0	.108	10	0	1	0	5	0	1
78		min	20.127	15	0	1	-11.595	4	0	1	0	2	0	1
79	2	max	190.667	1	0	1	.108	10	0	1	0	10	0	1
80		min	20.147	15	0	1	-11.651	4	0	1	-.001	4	0	1
81	3	max	190.732	1	0	1	.108	10	0	1	0	10	0	1
82		min	20.166	15	0	1	-11.707	4	0	1	-.002	4	0	1
83	4	max	190.797	1	0	1	.108	10	0	1	0	10	0	1
84		min	20.186	15	0	1	-11.763	4	0	1	-.003	4	0	1
85	5	max	190.861	1	0	1	.108	10	0	1	0	10	0	1

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	20.205	15	0	1	-11.819	4	0	1	-.004	4	0	1
87		6	max	190.926	1	0	1	.108	10	0	1	0	10	0	1
88			min	20.225	15	0	1	-11.875	4	0	1	-.005	4	0	1
89		7	max	190.991	1	0	1	.108	10	0	1	0	10	0	1
90			min	20.244	15	0	1	-11.931	4	0	1	-.006	4	0	1
91		8	max	191.056	1	0	1	.108	10	0	1	0	10	0	1
92			min	20.264	15	0	1	-11.987	4	0	1	-.007	4	0	1
93		9	max	191.12	1	0	1	.108	10	0	1	0	10	0	1
94			min	20.283	15	0	1	-12.043	4	0	1	-.008	4	0	1
95		10	max	191.185	1	0	1	.108	10	0	1	0	10	0	1
96			min	20.303	15	0	1	-12.099	4	0	1	-.01	4	0	1
97		11	max	191.25	1	0	1	.108	10	0	1	0	10	0	1
98			min	20.323	15	0	1	-12.155	4	0	1	-.011	4	0	1
99		12	max	191.314	1	0	1	.108	10	0	1	0	10	0	1
100			min	20.342	15	0	1	-12.212	4	0	1	-.012	4	0	1
101		13	max	191.379	1	0	1	.108	10	0	1	0	10	0	1
102			min	20.362	15	0	1	-12.268	4	0	1	-.013	4	0	1
103		14	max	191.444	1	0	1	.108	10	0	1	0	10	0	1
104			min	20.381	15	0	1	-12.324	4	0	1	-.014	4	0	1
105		15	max	191.508	1	0	1	.108	10	0	1	0	10	0	1
106			min	20.401	15	0	1	-12.38	4	0	1	-.015	4	0	1
107		16	max	191.573	1	0	1	.108	10	0	1	0	10	0	1
108			min	20.42	15	0	1	-12.436	4	0	1	-.016	4	0	1
109		17	max	191.638	1	0	1	.108	10	0	1	0	10	0	1
110			min	20.44	15	0	1	-12.492	4	0	1	-.017	4	0	1
111		18	max	191.703	1	0	1	.108	10	0	1	0	10	0	1
112			min	20.459	15	0	1	-12.548	4	0	1	-.018	4	0	1
113		19	max	191.767	1	0	1	.108	10	0	1	0	10	0	1
114			min	20.479	15	0	1	-12.604	4	0	1	-.019	4	0	1
115	M6	1	max	530.618	2	.655	6	1.005	4	0	3	0	3	0	1
116			min	-961.677	3	.143	15	-.317	3	0	5	0	1	0	1
117		2	max	530.753	2	.597	6	.882	4	0	3	0	3	0	15
118			min	-961.575	3	.129	15	-.317	3	0	5	0	1	0	6
119		3	max	530.888	2	.54	6	.759	4	0	3	0	4	0	15
120			min	-961.474	3	.116	15	-.317	3	0	5	0	1	0	6
121		4	max	531.023	2	.483	6	.636	4	0	3	0	4	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	532.507	2	-.016	2	.001	9	0	3	0	4	0	12
144		min	-960.26	3	-.188	3	-.72	5	0	5	0	3	0	2
145	16	max	532.641	2	-.06	15	.001	9	0	3	0	4	0	12
146		min	-960.159	3	-.221	3	-.844	5	0	5	0	3	0	2
147	17	max	532.776	2	-.073	15	.001	9	0	3	0	4	0	3
148		min	-960.058	3	-.266	4	-.967	5	0	5	0	3	0	2
149	18	max	532.911	2	-.087	15	.001	9	0	3	0	9	0	3
150		min	-959.957	3	-.323	4	-1.09	5	0	5	0	3	0	2
151	19	max	533.046	2	-.1	15	.001	9	0	3	0	9	0	3
152		min	-959.856	3	-.381	4	-1.213	5	0	5	0	3	0	2
153	M7	1	max	659.86	2	1.761	.058	3	0	9	0	4	0	2
154		min	-555.715	3	.424	15	-1.291	4	0	3	0	3	0	3
155	2	max	659.79	2	1.584	4	.058	3	0	9	0	4	0	2
156		min	-555.767	3	.382	15	-1.157	4	0	3	0	3	0	3
157	3	max	659.72	2	1.408	4	.058	3	0	9	0	9	0	2
158		min	-555.82	3	.341	15	-1.024	4	0	3	0	3	0	3
159	4	max	659.65	2	1.232	4	.058	3	0	9	0	9	0	15
160		min	-555.872	3	.299	15	-.89	4	0	3	0	3	0	3
161	5	max	659.58	2	1.055	4	.058	3	0	9	0	9	0	15
162		min	-555.925	3	.258	15	-.756	4	0	3	0	5	0	3
163	6	max	659.51	2	.879	4	.058	3	0	9	0	9	0	15
164		min	-555.977	3	.216	15	-.623	4	0	3	0	5	0	6
165	7	max	659.44	2	.702	4	.058	3	0	9	0	9	0	15
166		min	-556.03	3	.175	15	-.489	4	0	3	0	5	0	6
167	8	max	659.37	2	.526	4	.058	3	0	9	0	9	0	15
168		min	-556.082	3	.133	15	-.355	4	0	3	0	5	-.001	6
169	9	max	659.3	2	.35	4	.058	3	0	9	0	9	0	15
170		min	-556.135	3	.071	12	-.222	4	0	3	-.001	5	-.001	6
171	10	max	659.23	2	.191	2	.058	3	0	9	0	9	0	15
172		min	-556.187	3	-.002	3	-.088	4	0	3	-.001	5	-.001	6
173	11	max	659.16	2	.054	2	.058	3	0	9	0	9	0	15
174		min	-556.24	3	-.105	3	-.002	9	0	3	-.001	5	-.001	6
175	12	max	659.09	2	-.032	15	.18	5	0	9	0	9	0	15
176		min	-556.292	3	-.208	3	-.002	9	0	3	-.001	5	-.001	6
177	13	max	659.02	2	-.074	15	.314	5	0	9	0	9	0	15
178		min	-556.345	3	-.357	6	-.002	9	0	3	0	5	-.001	6
179	14	max	658.95	2	-.115	15	.448	5	0	9	0	9	0	15
180		min	-556.397	3	-.533	6	-.002	9	0	3	0	5	-.001	6
181	15	max	658.88	2	-.157	15	.581	5	0	9	0	9	0	15
182		min	-556.45	3	-.709	6	-.002	9	0	3	0	5	0	6
183	16	max	658.81	2	-.198	15	.715	5	0	9	0	9	0	15
184		min	-556.502	3	-.886	6	-.002	9	0	3	0	5	0	6
185	17	max	658.74	2	-.24	15	.849	5	0	9	0	9	0	15
186		min	-556.555	3	-1.062	6	-.002	9	0	3	0	5	0	6
187	18	max	658.67	2	-.281	15	.982	5	0	9	0	9	0	15
188		min	-556.607	3	-1.239	6	-.002	9	0	3	0	5	0	6
189	19	max	658.6	2	-.323	15	1.116	5	0	9	0	9	0	1
190		min	-556.66	3	-1.415	6	-.002	9	0	3	0	3	0	1
191	M8	1	max	554.016	2	0	.026	9	0	1	0	4	0	1
192		min	13.514	15	0	1	-11.831	4	0	1	0	3	0	1
193	2	max	554.081	2	0	1	.026	9	0	1	0	9	0	1
194		min	13.534	15	0	1	-11.887	4	0	1	-.001	4	0	1
195	3	max	554.145	2	0	1	.026	9	0	1	0	9	0	1
196		min	13.553	15	0	1	-11.943	4	0	1	-.002	4	0	1
197	4	max	554.21	2	0	1	.026	9	0	1	0	9	0	1
198		min	13.573	15	0	1	-11.999	4	0	1	-.003	4	0	1
199	5	max	554.275	2	0	1	.026	9	0	1	0	9	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	13.592	15	0	1	-12.055	4	0	1	-.004	4	0	1
201		6	max	554.34	2	0	1	.026	9	0	1	0	9	0	1
202			min	13.612	15	0	1	-12.111	4	0	1	-.005	4	0	1
203		7	max	554.404	2	0	1	.026	9	0	1	0	9	0	1
204			min	13.631	15	0	1	-12.167	4	0	1	-.006	4	0	1
205		8	max	554.469	2	0	1	.026	9	0	1	0	9	0	1
206			min	13.651	15	0	1	-12.223	4	0	1	-.008	4	0	1
207		9	max	554.534	2	0	1	.026	9	0	1	0	9	0	1
208			min	13.67	15	0	1	-12.279	4	0	1	-.009	4	0	1
209		10	max	554.598	2	0	1	.026	9	0	1	0	9	0	1
210			min	13.69	15	0	1	-12.335	4	0	1	-.01	4	0	1
211		11	max	554.663	2	0	1	.026	9	0	1	0	9	0	1
212			min	13.709	15	0	1	-12.392	4	0	1	-.011	4	0	1
213		12	max	554.728	2	0	1	.026	9	0	1	0	9	0	1
214			min	13.729	15	0	1	-12.448	4	0	1	-.012	4	0	1
215		13	max	554.793	2	0	1	.026	9	0	1	0	9	0	1
216			min	13.748	15	0	1	-12.504	4	0	1	-.013	4	0	1
217		14	max	554.857	2	0	1	.026	9	0	1	0	9	0	1
218			min	13.768	15	0	1	-12.56	4	0	1	-.014	4	0	1
219		15	max	554.922	2	0	1	.026	9	0	1	0	9	0	1
220			min	13.787	15	0	1	-12.616	4	0	1	-.015	4	0	1
221		16	max	554.987	2	0	1	.026	9	0	1	0	9	0	1
222			min	13.807	15	0	1	-12.672	4	0	1	-.016	4	0	1
223		17	max	555.051	2	0	1	.026	9	0	1	0	9	0	1
224			min	13.827	15	0	1	-12.728	4	0	1	-.018	4	0	1
225		18	max	555.116	2	0	1	.026	9	0	1	0	9	0	1
226			min	13.846	15	0	1	-12.784	4	0	1	-.019	4	0	1
227		19	max	555.181	2	0	1	.026	9	0	1	0	9	0	1
228			min	13.866	15	0	1	-12.84	4	0	1	-.02	4	0	1
229	M10	1	max	183.242	2	.715	4	1.089	5	0	1	0	9	0	1
230			min	-228.435	3	.184	15	-.036	9	0	5	0	3	0	1
231		2	max	183.377	2	.658	4	.965	5	0	1	0	4	0	15
232			min	-228.334	3	.17	15	-.036	9	0	5	0	3	0	4
233		3	max	183.512	2	.6	4	.842	5	0	1	0	4	0	15
234			min	-228.233	3	.157	15	-.036	9	0	5	0	3	0	4
235		4	max	183.647	2	.543	4	.719	5	0	1	0	4	0	15
236			min	-228.132	3	.143	15	-.036	9	0	5	0	3	0	4
237		5	max	183.782	2	.485	4	.596	5	0	1	0	4	0	15
238			min	-228.031	3	.13	15	-.036	9	0	5	0	3	0	4
239		6	max	183.916	2	.428	4	.473	5	0	1	0	4	0	15
240			min	-227.93	3	.116	15	-.036	9	0	5	0	3	0	4
241		7	max	184.051	2	.37	4	.35	5	0	1	0	4	0	15
242			min	-227.829	3	.103	15	-.036	9	0	5	0	3	0	4
243		8	max	184.186	2	.313	4	.227	5	0	1	0	5	0	15
244			min	-227.727	3	.083	12	-.036	9	0	5	0	3	0	4
245		9	max	184.321	2	.255	4	.103	5	0	1	0	5	0	15
246			min	-227.626	3	.06	12	-.036	9	0	5	0	3	0	4
247		10	max	184.456	2	.198	4	.011	10	0	1	0	5	0	15
248			min	-227.525	3	.038	12	-.037	14	0	5	0	3	0	4
249		11	max	184.591	2	.14	4	.011	10	0	1	0	5	0	15
250			min	-227.424	3	.015	12	-.154	4	0	5	0	3	0	4
251		12	max	184.726	2	.083	4	.011	10	0	1	0	5	0	15
252			min	-227.323	3	-.013	3	-.277	4	0	5	0	3	0	4
253		13	max	184.86	2	.032	5	.011	10	0	1	0	5	0	15
254			min	-227.222	3	-.047	3	-.401	4	0	5	0	3	0	4
255		14	max	184.995	2	.011	5	.011	10	0	1	0	5	0	12
256			min	-227.12	3	-.08	3	-.524	4	0	5	0	3	0	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	185.13	2	-.005	15	.011	10	0	1	0	5	0	12
258		min	-227.019	3	-.114	3	-.647	4	0	5	0	3	0	4
259	16	max	185.265	2	-.019	15	.011	10	0	1	0	5	0	12
260		min	-226.918	3	-.149	6	-.77	4	0	5	0	3	0	4
261	17	max	185.4	2	-.032	15	.011	10	0	1	0	5	0	12
262		min	-226.817	3	-.206	6	-.893	4	0	5	0	3	0	4
263	18	max	185.535	2	-.046	15	.011	10	0	1	0	5	0	12
264		min	-226.716	3	-.264	6	-1.016	4	0	5	0	3	0	4
265	19	max	185.67	2	-.059	15	.011	10	0	1	0	10	0	12
266		min	-226.615	3	-.321	6	-1.14	4	0	5	0	3	0	4
267	M11	1	max	244.233	2	1.719	.054	1	0	3	0	3	0	2
268		min	-226.437	3	.396	15	-1.269	5	0	10	0	1	0	15
269	2	max	244.163	2	1.542	6	.054	1	0	3	0	3	0	2
270		min	-226.49	3	.354	15	-1.135	5	0	10	0	1	0	15
271	3	max	244.093	2	1.366	6	.054	1	0	3	0	3	0	2
272		min	-226.542	3	.313	15	-1.002	5	0	10	0	1	0	4
273	4	max	244.023	2	1.189	6	.054	1	0	3	0	3	0	15
274		min	-226.595	3	.271	15	-.868	5	0	10	0	4	0	4
275	5	max	243.953	2	1.013	6	.054	1	0	3	0	3	0	15
276		min	-226.647	3	.23	15	-.734	5	0	10	0	4	0	4
277	6	max	243.883	2	.837	6	.054	1	0	3	0	3	0	15
278		min	-226.7	3	.188	15	-.601	5	0	10	0	4	0	4
279	7	max	243.813	2	.66	6	.054	1	0	3	0	3	0	15
280		min	-226.752	3	.147	15	-.467	5	0	10	0	4	-.001	4
281	8	max	243.743	2	.484	6	.054	1	0	3	0	3	0	15
282		min	-226.805	3	.106	15	-.333	5	0	10	0	4	-.001	4
283	9	max	243.673	2	.308	6	.054	1	0	3	0	3	0	15
284		min	-226.857	3	.064	15	-.2	5	0	10	0	4	-.001	4
285	10	max	243.603	2	.144	2	.054	1	0	3	0	3	0	15
286		min	-226.91	3	.023	15	-.071	3	0	10	0	4	-.001	4
287	11	max	243.533	2	.007	2	.085	4	0	3	0	3	0	15
288		min	-226.962	3	-.046	4	-.071	3	0	10	0	4	-.001	4
289	12	max	243.463	2	-.06	15	.219	4	0	3	0	3	0	15
290		min	-227.015	3	-.222	4	-.071	3	0	10	0	4	-.001	4
291	13	max	243.393	2	-.102	15	.353	4	0	3	0	3	0	15
292		min	-227.067	3	-.399	4	-.071	3	0	10	0	4	-.001	4
293	14	max	243.323	2	-.143	15	.486	4	0	3	0	3	0	15
294		min	-227.12	3	-.575	4	-.071	3	0	10	0	4	-.001	4
295	15	max	243.253	2	-.185	15	.62	4	0	3	0	3	0	15
296		min	-227.172	3	-.751	4	-.071	3	0	10	0	4	0	4
297	16	max	243.183	2	-.226	15	.754	4	0	3	0	3	0	15
298		min	-227.225	3	-.928	4	-.071	3	0	10	0	4	0	4
299	17	max	243.113	2	-.268	15	.887	4	0	3	0	3	0	15
300		min	-227.277	3	-1.104	4	-.071	3	0	10	0	5	0	4
301	18	max	243.043	2	-.309	15	1.021	4	0	3	0	3	0	15
302		min	-227.33	3	-1.281	4	-.071	3	0	10	0	10	0	4
303	19	max	242.973	2	-.35	15	1.155	4	0	3	0	3	0	1
304		min	-227.382	3	-1.457	4	-.071	3	0	10	0	10	0	1
305	M12	1	max	190.989	1	0	.402	3	0	1	0	4	0	1
306		min	-2.569	5	0	1	-10.943	5	0	1	0	3	0	1
307	2	max	191.054	1	0	1	.402	3	0	1	0	1	0	1
308		min	-2.539	5	0	1	-10.999	5	0	1	0	5	0	1
309	3	max	191.118	1	0	1	.402	3	0	1	0	3	0	1
310		min	-2.508	5	0	1	-11.055	5	0	1	-.002	5	0	1
311	4	max	191.183	1	0	1	.402	3	0	1	0	3	0	1
312		min	-2.478	5	0	1	-11.111	5	0	1	-.003	5	0	1
313	5	max	191.248	1	0	1	.402	3	0	1	0	3	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-2.448	5	0	1	-11.167	5	0	1	-.004	5	0	1
315	6	max	191.312	1	0	1	.402	3	0	1	0	3	0	1
316		min	-2.418	5	0	1	-11.223	5	0	1	-.005	5	0	1
317	7	max	191.377	1	0	1	.402	3	0	1	0	3	0	1
318		min	-2.388	5	0	1	-11.279	5	0	1	-.006	5	0	1
319	8	max	191.442	1	0	1	.402	3	0	1	0	3	0	1
320		min	-2.357	5	0	1	-11.335	5	0	1	-.007	5	0	1
321	9	max	191.507	1	0	1	.402	3	0	1	0	3	0	1
322		min	-2.327	5	0	1	-11.392	5	0	1	-.008	5	0	1
323	10	max	191.571	1	0	1	.402	3	0	1	0	3	0	1
324		min	-2.297	5	0	1	-11.448	5	0	1	-.009	5	0	1
325	11	max	191.636	1	0	1	.402	3	0	1	0	3	0	1
326		min	-2.267	5	0	1	-11.504	5	0	1	-.01	5	0	1
327	12	max	191.701	1	0	1	.402	3	0	1	0	3	0	1
328		min	-2.237	5	0	1	-11.56	5	0	1	-.011	5	0	1
329	13	max	191.765	1	0	1	.402	3	0	1	0	3	0	1
330		min	-2.206	5	0	1	-11.616	5	0	1	-.012	5	0	1
331	14	max	191.83	1	0	1	.402	3	0	1	0	3	0	1
332		min	-2.176	5	0	1	-11.672	5	0	1	-.013	5	0	1
333	15	max	191.895	1	0	1	.402	3	0	1	0	3	0	1
334		min	-2.146	5	0	1	-11.728	5	0	1	-.014	5	0	1
335	16	max	191.96	1	0	1	.402	3	0	1	0	3	0	1
336		min	-2.116	5	0	1	-11.784	5	0	1	-.015	5	0	1
337	17	max	192.024	1	0	1	.402	3	0	1	0	3	0	1
338		min	-2.086	5	0	1	-11.84	5	0	1	-.016	5	0	1
339	18	max	192.089	1	0	1	.402	3	0	1	0	3	0	1
340		min	-2.055	5	0	1	-11.896	5	0	1	-.017	5	0	1
341	19	max	192.154	1	0	1	.402	3	0	1	0	3	0	1
342		min	-2.025	5	0	1	-11.952	5	0	1	-.018	5	0	1
343	M1	1	max	54.073	1	335.11	3	2.723	10	0	.023	4	0	2
344		min	1.401	10	-204.807	2	-13.328	4	0	3	-.005	10	0	3
345	2	max	54.233	1	334.938	3	2.723	10	0	2	.02	4	.045	2
346		min	1.534	10	-205.036	2	-13.086	4	0	3	-.005	10	-.073	3
347	3	max	120.234	3	4.28	4	2.712	10	0	10	.017	4	.089	2
348		min	-36.38	2	-30.535	2	-11.777	4	0	1	-.004	10	-.144	3
349	4	max	120.354	3	3.987	4	2.712	10	0	10	.014	4	.095	2
350		min	-36.22	2	-30.763	2	-11.535	4	0	1	-.004	10	-.143	3
351	5	max	120.475	3	3.693	4	2.712	10	0	10	.012	4	.102	2
352		min	-36.06	2	-30.992	2	-11.293	4	0	1	-.003	10	-.141	3
353	6	max	120.595	3	3.4	4	2.712	10	0	10	.009	4	.109	2
354		min	-35.9	2	-31.221	2	-11.051	4	0	1	-.002	10	-.14	3
355	7	max	120.715	3	3.106	4	2.712	10	0	10	.007	4	.115	2
356		min	-35.739	2	-31.449	2	-10.809	4	0	1	-.002	10	-.138	3
357	8	max	120.835	3	2.813	4	2.712	10	0	10	.005	3	.122	2
358		min	-35.579	2	-31.678	2	-10.567	4	0	1	-.001	10	-.136	3
359	9	max	120.955	3	2.519	4	2.712	10	0	10	.004	3	.129	2
360		min	-35.419	2	-31.907	2	-10.325	4	0	1	0	10	-.135	3
361	10	max	121.075	3	2.255	14	2.712	10	0	10	.002	3	.136	2
362		min	-35.259	2	-32.136	2	-10.083	4	0	1	0	5	-.133	3
363	11	max	121.195	3	2.03	14	2.712	10	0	10	0	3	.143	2
364		min	-35.099	2	-32.364	2	-9.841	4	0	1	-.002	4	-.131	3
365	12	max	121.315	3	1.805	14	2.712	10	0	10	.001	10	.15	2
366		min	-34.939	2	-32.593	2	-9.599	4	0	1	-.004	4	-.129	3
367	13	max	121.436	3	1.581	14	2.712	10	0	10	.002	10	.157	2
368		min	-34.778	2	-32.822	2	-9.357	4	0	1	-.006	4	-.128	3
369	14	max	121.556	3	1.356	14	2.712	10	0	10	.002	10	.164	2
370		min	-34.618	2	-33.051	2	-9.115	4	0	1	-.008	4	-.126	3



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	121.676	3	1.131	14	2.712	10	0	10	.003	10	.172	2
372		min	-34.458	2	-33.279	2	-8.873	4	0	1	-.01	4	-.124	3
373	16	max	80.907	2	181.139	2	2.73	10	0	9	.004	10	.177	2
374		min	2.594	15	-209.099	3	-7.975	14	0	5	-.012	4	-.12	3
375	17	max	81.068	2	180.911	2	2.73	10	0	9	.004	10	.138	2
376		min	2.642	15	-209.271	3	-7.853	14	0	5	-.014	4	-.075	3
377	18	max	-1.559	10	317.741	2	2.838	10	0	5	.005	10	.069	2
378		min	-54.218	1	-171.4	3	-18.503	4	0	2	-.018	4	-.038	3
379	19	max	-1.425	10	317.512	2	2.838	10	0	5	.005	10	0	2
380		min	-54.058	1	-171.572	3	-18.261	4	0	2	-.022	4	0	3
381	M5	1	max	152.478	1	1010.319	3	0	1	0	.024	4	0	3
382		min	-17.086	3	-598.261	2	-98.728	3	0	3	0	11	0	2
383	2	max	152.638	1	1010.148	3	0	1	0	9	.021	4	.129	2
384		min	-16.966	3	-598.49	2	-98.728	3	0	3	-.005	3	-.218	3
385	3	max	295.493	3	4.136	9	10.322	3	0	3	.017	4	.257	2
386		min	-72.172	2	-90.484	2	-13.423	4	0	4	-.026	3	-.433	3
387	4	max	295.613	3	3.946	9	10.322	3	0	3	.014	4	.277	2
388		min	-72.012	2	-90.713	2	-13.181	4	0	4	-.023	3	-.425	3
389	5	max	295.733	3	3.755	9	10.322	3	0	3	.011	4	.296	2
390		min	-71.852	2	-90.941	2	-12.939	4	0	4	-.021	3	-.417	3
391	6	max	295.854	3	3.564	9	10.322	3	0	3	.008	4	.316	2
392		min	-71.692	2	-91.17	2	-12.697	4	0	4	-.019	3	-.409	3
393	7	max	295.974	3	3.374	9	10.322	3	0	3	.006	4	.336	2
394		min	-71.532	2	-91.399	2	-12.455	4	0	4	-.017	3	-.401	3
395	8	max	296.094	3	3.183	9	10.322	3	0	3	.003	4	.356	2
396		min	-71.371	2	-91.628	2	-12.213	4	0	4	-.014	3	-.394	3
397	9	max	296.214	3	2.993	9	10.322	3	0	3	0	4	.376	2
398		min	-71.211	2	-91.856	2	-11.971	4	0	4	-.012	3	-.386	3
399	10	max	296.334	3	2.802	9	10.322	3	0	3	0	1	.396	2
400		min	-71.051	2	-92.085	2	-11.729	4	0	4	-.01	3	-.378	3
401	11	max	296.454	3	2.611	9	10.322	3	0	3	0	1	.416	2
402		min	-70.891	2	-92.314	2	-11.487	4	0	4	-.008	3	-.37	3
403	12	max	296.574	3	2.421	9	10.322	3	0	3	0	1	.436	2
404		min	-70.731	2	-92.543	2	-11.245	4	0	4	-.007	4	-.362	3
405	13	max	296.694	3	2.23	9	10.322	3	0	3	0	1	.456	2
406		min	-70.571	2	-92.771	2	-11.003	4	0	4	-.01	4	-.354	3
407	14	max	296.815	3	2.039	9	10.322	3	0	3	0	1	.476	2
408		min	-70.41	2	-93	2	-10.761	4	0	4	-.012	4	-.345	3
409	15	max	296.935	3	1.849	9	10.322	3	0	3	.001	3	.496	2
410		min	-70.25	2	-93.229	2	-10.519	4	0	4	-.014	4	-.337	3
411	16	max	239.779	2	490.118	2	10.311	3	0	3	.003	3	.511	2
412		min	-1.644	5	-530.116	3	-9.167	4	0	4	-.017	4	-.325	3
413	17	max	239.939	2	489.889	2	10.311	3	0	3	.005	3	.405	2
414		min	-1.569	5	-530.287	3	-8.925	4	0	4	-.019	4	-.21	3
415	18	max	4.668	3	939.481	2	9.428	3	0	4	.007	3	.203	2
416		min	-152.668	1	-484.673	3	-20.347	5	0	9	-.023	4	-.105	3
417	19	max	4.788	3	939.252	2	9.428	3	0	4	.009	3	0	3
418		min	-152.508	1	-484.845	3	-20.105	5	0	9	-.027	4	0	2
419	M9	1	max	54.073	1	334.965	3	105.289	3	0	.005	10	0	2
420		min	.764	15	-204.807	2	-2.723	10	0	2	-.028	3	0	3
421	2	max	54.233	1	334.793	3	105.289	3	0	3	.018	5	.045	2
422		min	.813	15	-205.036	2	-2.723	10	0	2	-.013	1	-.073	3
423	3	max	119.288	3	3.263	9	7.198	1	0	1	.035	5	.089	2
424		min	-35.977	2	-30.509	2	-17.117	5	0	5	-.011	1	-.144	3
425	4	max	119.408	3	3.073	9	7.198	1	0	1	.031	5	.095	2
426		min	-35.816	2	-30.738	2	-16.875	5	0	5	-.009	1	-.143	3
427	5	max	119.528	3	2.882	9	7.198	1	0	1	.028	5	.102	2



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-35.656	2	-30.966	2	-16.633	5	0	5	-.008	1	-.141	3
429	6	max	119.648	3	2.692	9	7.198	1	0	1	.024	5	.109	2
430		min	-35.496	2	-31.195	2	-16.391	5	0	5	-.006	1	-.14	3
431	7	max	119.768	3	2.501	9	7.198	1	0	1	.021	5	.115	2
432		min	-35.336	2	-31.424	2	-16.149	5	0	5	-.005	1	-.138	3
433	8	max	119.889	3	2.31	9	7.198	1	0	1	.017	5	.122	2
434		min	-35.176	2	-31.652	2	-15.907	5	0	5	-.003	1	-.136	3
435	9	max	120.009	3	2.12	9	7.198	1	0	1	.014	5	.129	2
436		min	-35.016	2	-31.881	2	-15.665	5	0	5	-.002	1	-.135	3
437	10	max	120.129	3	1.929	9	7.198	1	0	1	.013	3	.136	2
438		min	-34.855	2	-32.11	2	-15.423	5	0	5	0	1	-.133	3
439	11	max	120.249	3	1.739	9	7.198	1	0	1	.012	3	.143	2
440		min	-34.695	2	-32.339	2	-15.181	5	0	5	0	10	-.131	3
441	12	max	120.369	3	1.548	9	7.198	1	0	1	.012	3	.15	2
442		min	-34.535	2	-32.567	2	-14.939	5	0	5	-.001	10	-.13	3
443	13	max	120.489	3	1.357	9	7.198	1	0	1	.011	3	.157	2
444		min	-34.375	2	-32.796	2	-14.697	5	0	5	-.002	10	-.128	3
445	14	max	120.609	3	1.167	9	7.198	1	0	1	.011	3	.164	2
446		min	-34.215	2	-33.025	2	-14.455	5	0	5	-.003	5	-.126	3
447	15	max	120.729	3	.976	9	7.198	1	0	1	.01	3	.172	2
448		min	-34.055	2	-33.254	2	-14.213	5	0	5	-.006	5	-.124	3
449	16	max	81.129	2	180.824	2	7.245	1	0	10	.01	3	.177	2
450		min	4.56	15	-210.093	3	-12.832	5	0	4	-.008	5	-.12	3
451	17	max	81.289	2	180.595	2	7.245	1	0	10	.011	1	.138	2
452		min	4.608	15	-210.265	3	-12.59	5	0	4	-.011	5	-.075	3
453	18	max	8.492	5	317.741	2	7.517	1	0	2	.013	1	.069	2
454		min	-54.218	1	-171.381	3	-23.296	5	0	3	-.016	5	-.038	3
455	19	max	8.567	5	317.512	2	7.517	1	0	2	.014	1	0	2
456		min	-54.058	1	-171.553	3	-23.054	5	0	3	-.021	5	0	3
457	M13	1	max	105.278	3	204.757	2	-.764	15	0	.028	3	0	2
458		min	-2.723	10	-335.05	3	-54.071	1	0	3	-.005	10	0	3
459	2	max	105.278	3	148.809	2	.404	10	0	2	.023	3	.096	3
460		min	-2.723	10	-241.691	3	-39.592	1	0	3	-.008	2	-.059	2
461	3	max	105.278	3	92.862	2	2.21	10	0	2	.018	3	.161	3
462		min	-2.723	10	-148.333	3	-25.113	1	0	3	-.012	1	-.099	2
463	4	max	105.278	3	36.914	2	4.016	10	0	2	.013	3	.195	3
464		min	-2.723	10	-54.975	3	-13.677	3	0	3	-.018	1	-.121	2
465	5	max	105.278	3	38.384	3	9.129	2	0	2	.009	3	.198	3
466		min	-2.723	10	-19.034	2	-12.726	3	0	3	-.019	1	-.124	2
467	6	max	105.278	3	131.742	3	18.325	1	0	2	.005	3	.169	3
468		min	-2.723	10	-74.982	2	-11.775	3	0	3	-.016	1	-.108	2
469	7	max	105.278	3	225.101	3	32.804	1	0	2	.005	5	.11	3
470		min	-2.723	10	-130.929	2	-10.824	3	0	3	-.007	1	-.074	2
471	8	max	105.278	3	318.459	3	47.283	1	0	2	.01	2	.019	3
472		min	-2.723	10	-186.877	2	-9.873	3	0	3	-.003	3	-.021	2
473	9	max	105.278	3	411.817	3	61.762	1	0	2	.024	1	.051	2
474		min	-2.723	10	-242.825	2	-8.922	3	0	3	-.006	3	-.102	3
475	10	max	105.278	3	-5.205	15	76.241	1	0	2	.047	1	.141	2
476		min	-2.723	10	-505.176	3	5.118	15	0	3	-.025	3	-.255	3
477	11	max	39.506	4	242.825	2	10.117	3	0	3	.024	1	.051	2
478		min	-2.723	10	-411.817	3	-61.762	1	0	2	-.022	3	-.102	3
479	12	max	36.232	4	186.877	2	11.068	3	0	3	.01	2	.019	3
480		min	-2.723	10	-318.459	3	-47.283	1	0	2	-.018	3	-.021	2
481	13	max	32.958	4	130.929	2	12.019	3	0	3	.003	10	.11	3
482		min	-2.723	10	-225.1	3	-32.804	1	0	2	-.014	3	-.074	2
483	14	max	29.684	4	74.982	2	12.97	3	0	3	0	10	.169	3
484		min	-2.723	10	-131.742	3	-18.324	1	0	2	-.016	1	-.108	2

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	26.411	4	19.034	2	13.922	3	0	3	0	5	.198	3
486			min	-2.723	10	-38.384	3	-9.128	2	0	2	-.019	1	-.124	2
487		16	max	23.137	4	54.975	3	15.391	4	0	3	.005	5	.195	3
488			min	-2.723	10	-36.914	2	-4.015	10	0	2	-.018	1	-.121	2
489		17	max	19.863	4	148.333	3	25.113	1	0	3	.009	5	.161	3
490			min	-2.723	10	-92.862	2	-2.21	10	0	2	-.012	1	-.099	2
491		18	max	16.589	4	241.692	3	39.592	1	0	3	.014	4	.096	3
492			min	-2.723	10	-148.809	2	-.404	10	0	2	-.008	2	-.059	2
493		19	max	13.315	4	335.05	3	54.071	1	0	3	.023	4	0	2
494			min	-2.723	10	-204.757	2	1.401	10	0	2	-.005	10	0	3
495	M16	1	max	23.047	5	317.579	2	8.567	5	0	3	.014	1	0	2
496			min	-7.508	1	-171.596	3	-54.06	1	0	2	-.021	5	0	3
497		2	max	19.773	5	230.059	2	9.578	5	0	3	.005	3	.05	3
498			min	-7.508	1	-125.596	3	-39.581	1	0	2	-.018	5	-.091	2
499		3	max	16.499	5	142.539	2	10.589	5	0	3	.001	3	.084	3
500			min	-7.508	1	-79.596	3	-25.102	1	0	2	-.017	4	-.153	2
501		4	max	13.225	5	55.019	2	11.601	5	0	3	-.001	12	.103	3
502			min	-7.508	1	-33.596	3	-10.623	1	0	2	-.018	1	-.186	2
503		5	max	9.951	5	12.404	3	12.612	5	0	3	-.002	10	.106	3
504			min	-7.508	1	-32.501	2	-8.293	3	0	2	-.019	1	-.19	2
505		6	max	6.677	5	58.404	3	18.335	1	0	3	0	10	.094	3
506			min	-7.508	1	-120.021	2	-7.342	3	0	2	-.016	1	-.165	2
507		7	max	3.403	5	104.404	3	32.815	1	0	3	.003	10	.067	3
508			min	-7.508	1	-207.541	2	-6.391	3	0	2	-.01	3	-.11	2
509		8	max	2.838	10	150.404	3	47.294	1	0	3	.01	2	.025	3
510			min	-7.508	1	-295.061	2	-5.44	3	0	2	-.012	3	-.026	2
511		9	max	2.838	10	196.404	3	61.773	1	0	3	.025	1	.087	2
512			min	-7.508	1	-382.581	2	-4.488	3	0	2	-.014	3	-.033	3
513		10	max	13.691	5	242.404	3	76.252	1	0	14	.048	1	.229	2
514			min	-8.757	14	-470.101	2	-3.537	3	0	2	-.015	3	-.106	3
515		11	max	10.418	5	382.581	2	5.159	5	0	2	.025	1	.087	2
516			min	-7.508	1	-196.404	3	-61.773	1	0	3	-.007	5	-.033	3
517		12	max	7.144	5	295.061	2	6.17	5	0	2	.01	2	.025	3
518			min	-7.508	1	-150.404	3	-47.294	1	0	3	-.006	5	-.026	2
519		13	max	3.87	5	207.541	2	7.181	5	0	2	.003	10	.067	3
520			min	-7.508	1	-104.404	3	-32.814	1	0	3	-.007	1	-.11	2
521		14	max	2.838	10	120.021	2	8.193	5	0	2	0	10	.094	3
522			min	-7.508	1	-58.404	3	-18.335	1	0	3	-.016	1	-.165	2
523		15	max	2.838	10	32.501	2	9.442	4	0	2	.002	5	.106	3
524			min	-7.508	1	-12.404	3	-9.099	2	0	3	-.019	1	-.19	2
525		16	max	2.838	10	33.596	3	13.762	4	0	2	.005	5	.103	3
526			min	-8.571	14	-55.019	2	-3.992	10	0	3	-.018	1	-.186	2
527		17	max	2.838	10	79.596	3	25.102	1	0	2	.009	5	.084	3
528			min	-11.727	4	-142.539	2	-2.187	10	0	3	-.012	1	-.153	2
529		18	max	2.838	10	125.596	3	39.581	1	0	2	.014	4	.05	3
530			min	-15	4	-230.059	2	-.381	10	0	3	-.008	2	-.091	2
531		19	max	2.838	10	171.596	3	54.06	1	0	2	.022	4	0	2
532			min	-18.274	4	-317.579	2	1.425	10	0	3	-.005	10	0	5
533	M15	1	max	0	1	.648	3	.184	3	0	1	0	1	0	1
534			min	-168.589	3	0	1	0	1	0	3	0	3	0	1
535		2	max	0	1	.576	3	.184	3	0	1	0	1	0	1
536			min	-168.664	3	0	1	0	1	0	3	0	3	0	3
537		3	max	0	1	.504	3	.184	3	0	1	0	1	0	1
538			min	-168.74	3	0	1	0	1	0	3	0	3	0	3
539		4	max	0	1	.432	3	.184	3	0	1	0	1	0	1
540			min	-168.815	3	0	1	0	1	0	3	0	3	0	3
541		5	max	0	1	.36	3	.184	3	0	1	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-168.891	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.288	3	.184	3	0	1	0	1	0	1
544			min	-168.966	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.216	3	.184	3	0	1	0	3	0	1
546			min	-169.042	3	0	1	0	1	0	3	0	1	0	3
547		8	max	0	1	.144	3	.184	3	0	1	0	3	0	1
548			min	-169.117	3	0	1	0	1	0	3	0	1	0	3
549		9	max	0	1	.072	3	.184	3	0	1	0	3	0	1
550			min	-169.193	3	0	1	0	1	0	3	0	1	0	3
551		10	max	0	1	0	1	.184	3	0	1	0	3	0	1
552			min	-169.268	3	0	1	0	1	0	3	0	1	0	3
553		11	max	0	1	0	1	.184	3	0	1	0	3	0	1
554			min	-169.344	3	-.072	3	0	1	0	3	0	1	0	3
555		12	max	0	1	0	1	.184	3	0	1	0	3	0	1
556			min	-169.42	3	-.144	3	0	1	0	3	0	1	0	3
557		13	max	0	1	0	1	.184	3	0	1	0	3	0	1
558			min	-169.495	3	-.216	3	0	1	0	3	0	1	0	3
559		14	max	0	1	0	1	.184	3	0	1	0	3	0	1
560			min	-169.571	3	-.288	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.184	3	0	1	0	3	0	1
562			min	-169.646	3	-.36	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.184	3	0	1	0	3	0	1
564			min	-169.722	3	-.432	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.184	3	0	1	0	3	0	1
566			min	-169.797	3	-.504	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.184	3	0	1	0	3	0	1
568			min	-169.873	3	-.576	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.184	3	0	1	0	3	0	1
570			min	-169.948	3	-.648	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.845	4	.401	4	0	3	0	3	0	1
572			min	-167.225	3	0	1	-.078	3	0	4	0	4	0	1
573		2	max	0	1	1.64	4	.359	4	0	3	0	3	0	1
574			min	-167.149	3	0	1	-.078	3	0	4	0	4	0	4
575		3	max	0	1	1.435	4	.317	4	0	3	0	3	0	1
576			min	-167.074	3	0	1	-.078	3	0	4	0	4	0	4
577		4	max	0	1	1.23	4	.275	4	0	3	0	3	0	1
578			min	-166.998	3	0	1	-.078	3	0	4	0	4	-.001	4
579		5	max	0	1	1.025	4	.233	4	0	3	0	3	0	1
580			min	-166.923	3	0	1	-.078	3	0	4	0	9	-.001	4
581		6	max	0	1	.82	4	.191	4	0	3	0	3	0	1
582			min	-166.847	3	0	1	-.078	3	0	4	0	9	-.002	4
583		7	max	0	1	.615	4	.149	4	0	3	0	3	0	1
584			min	-166.771	3	0	1	-.078	3	0	4	0	9	-.002	4
585		8	max	0	1	.41	4	.107	4	0	3	0	5	0	1
586			min	-166.696	3	0	1	-.078	3	0	4	0	9	-.002	4
587		9	max	0	1	.205	4	.065	4	0	3	0	5	0	1
588			min	-166.62	3	0	1	-.078	3	0	4	0	9	-.002	4
589		10	max	0	1	0	1	.023	4	0	3	0	5	0	1
590			min	-166.545	3	0	1	-.078	3	0	4	0	9	-.002	4
591		11	max	0	1	0	1	.009	9	0	3	0	5	0	1
592			min	-166.469	3	-.205	4	-.078	3	0	4	0	9	-.002	4
593		12	max	.046	9	0	1	.009	9	0	3	0	5	0	1
594			min	-166.394	3	-.41	4	-.078	3	0	4	0	9	-.002	4
595		13	max	.13	9	0	1	.009	9	0	3	0	5	0	1
596			min	-166.318	3	-.615	4	-.104	5	0	4	0	3	-.002	4
597		14	max	.214	9	0	1	.009	9	0	3	0	5	0	1
598			min	-166.243	3	-.82	4	-.146	5	0	4	0	3	-.002	4



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.298	9	0	1	.009	9	0	3	0	5	0	1
600		min	-166.167	3	-1.025	4	-.188	5	0	4	0	3	-.001	4
601	16	max	.381	9	0	1	.009	9	0	3	0	9	0	1
602		min	-166.092	3	-1.23	4	-.23	5	0	4	0	3	-.001	4
603	17	max	.465	9	0	1	.009	9	0	3	0	9	0	1
604		min	-166.016	3	-1.435	4	-.272	5	0	4	0	3	0	4
605	18	max	.549	9	0	1	.009	9	0	3	0	9	0	1
606		min	-165.941	3	-1.64	4	-.314	5	0	4	0	4	0	4
607	19	max	.633	9	0	1	.009	9	0	3	0	9	0	1
608		min	-165.865	3	-1.845	4	-.356	5	0	4	0	4	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	2	.011	2	0	9	6.422e-4	5	NC	3	NC	1
2			min	-.004	3	-.011	3	-.009	5	-2.53e-4	3	3924.883	2	NC	1
3		2	max	.002	2	.01	2	0	9	6.638e-4	5	NC	3	NC	1
4			min	-.004	3	-.011	3	-.009	5	-2.391e-4	3	4292.811	2	NC	1
5		3	max	.002	2	.009	2	0	9	6.854e-4	5	NC	3	NC	1
6			min	-.003	3	-.01	3	-.009	5	-2.252e-4	3	4732.079	2	NC	1
7		4	max	.002	2	.008	2	0	9	7.069e-4	5	NC	1	NC	1
8			min	-.003	3	-.01	3	-.009	5	-2.113e-4	3	5260.135	2	NC	1
9		5	max	.001	2	.007	2	0	9	7.285e-4	5	NC	1	NC	1
10			min	-.003	3	-.009	3	-.008	5	-1.974e-4	3	5900.294	2	NC	1
11		6	max	.001	2	.006	2	0	9	7.5e-4	5	NC	1	NC	1
12			min	-.003	3	-.009	3	-.008	5	-1.835e-4	3	6684.218	2	NC	1
13		7	max	.001	2	.006	2	0	9	7.716e-4	5	NC	1	NC	1
14			min	-.002	3	-.008	3	-.008	5	-1.696e-4	3	7655.721	2	NC	1
15		8	max	.001	2	.005	2	0	9	7.932e-4	5	NC	1	NC	1
16			min	-.002	3	-.008	3	-.007	5	-1.558e-4	3	8876.773	2	NC	1
17		9	max	.001	2	.004	2	0	9	8.147e-4	5	NC	1	NC	1
18			min	-.002	3	-.007	3	-.007	5	-1.419e-4	3	NC	1	NC	1
19		10	max	0	2	.003	2	0	9	8.363e-4	5	NC	1	NC	1
20		min	-.002	3	-.007	3	-.007	5	-1.28e-4	3	NC	1	NC	1	
21	11	max	0	2	.003	2	0	9	8.578e-4	5	NC	1	NC	1	
22		min	-.002	3	-.006	3	-.006	5	-1.141e-4	3	NC	1	NC	1	
23	12	max	0	2	.002	2	0	9	8.794e-4	5	NC	1	NC	1	
24		min	-.001	3	-.005	3	-.005	5	-1.002e-4	3	NC	1	NC	1	
25	13	max	0	2	.002	2	0	9	9.01e-4	5	NC	1	NC	1	
26		min	-.001	3	-.005	3	-.005	5	-8.631e-5	3	NC	1	NC	1	
27	14	max	0	2	.001	2	0	9	9.225e-4	5	NC	1	NC	1	
28		min	-.001	3	-.004	3	-.004	5	-7.242e-5	3	NC	1	NC	1	
29	15	max	0	2	0	2	0	9	9.441e-4	5	NC	1	NC	1	
30		min	0	3	-.003	3	-.003	5	-5.853e-5	3	NC	1	NC	1	
31	16	max	0	2	0	2	0	9	9.656e-4	5	NC	1	NC	1	
32		min	0	3	-.002	3	-.002	5	-4.464e-5	3	NC	1	NC	1	
33	17	max	0	2	0	2	0	9	9.872e-4	5	NC	1	NC	1	
34		min	0	3	-.002	3	-.002	5	-3.075e-5	3	NC	1	NC	1	
35	18	max	0	2	0	2	0	9	1.009e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-2.311e-5	9	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.03e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-1.779e-5	9	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	8.548e-6	9	NC	1	NC	1
40			min	0	1	0	1	0	1	-4.923e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.002	5	1.142e-5	9	NC	1	NC	1
42			min	0	2	0	3	0	9	-4.925e-4	5	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43	3	max	0	3	0	2	.005	5	1.437e-5	1	NC	1	NC	1
44		min	0	2	-.002	3	0	9	-4.927e-4	5	NC	1	NC	1
45	4	max	0	3	0	2	.007	4	1.761e-5	1	NC	1	NC	1
46		min	0	2	-.003	3	0	9	-4.929e-4	5	NC	1	NC	1
47	5	max	0	3	0	2	.01	4	2.085e-5	1	NC	1	NC	1
48		min	0	2	-.004	3	0	9	-4.931e-4	5	NC	1	NC	1
49	6	max	0	3	0	2	.012	4	2.408e-5	1	NC	1	NC	1
50		min	0	2	-.005	3	0	9	-4.933e-4	5	NC	1	NC	1
51	7	max	0	3	0	2	.015	4	2.732e-5	1	NC	1	NC	1
52		min	0	2	-.005	3	0	10	-4.935e-4	5	NC	1	NC	1
53	8	max	0	3	.001	2	.017	4	3.056e-5	1	NC	1	NC	1
54		min	-.001	2	-.006	3	0	10	-4.937e-4	5	NC	1	NC	1
55	9	max	.001	3	.001	2	.019	4	3.379e-5	1	NC	1	NC	1
56		min	-.001	2	-.007	3	0	10	-4.939e-4	5	NC	1	NC	1
57	10	max	.001	3	.002	2	.021	4	3.703e-5	1	NC	1	NC	1
58		min	-.001	2	-.007	3	0	10	-4.941e-4	5	NC	1	NC	1
59	11	max	.001	3	.002	2	.023	4	4.027e-5	1	NC	1	NC	1
60		min	-.002	2	-.008	3	0	10	-4.943e-4	5	NC	1	NC	1
61	12	max	.002	3	.003	2	.025	4	4.35e-5	1	NC	1	NC	1
62		min	-.002	2	-.008	3	0	10	-4.945e-4	5	NC	1	NC	1
63	13	max	.002	3	.004	2	.027	4	4.724e-5	3	NC	1	NC	1
64		min	-.002	2	-.008	3	0	10	-4.947e-4	5	NC	1	NC	1
65	14	max	.002	3	.004	2	.029	4	5.103e-5	3	NC	1	NC	1
66		min	-.002	2	-.009	3	0	10	-4.949e-4	5	NC	1	NC	1
67	15	max	.002	3	.005	2	.03	4	5.482e-5	3	NC	1	NC	1
68		min	-.002	2	-.009	3	0	10	-4.951e-4	5	8557.174	2	NC	1
69	16	max	.002	3	.006	2	.032	4	5.861e-5	3	NC	1	NC	1
70		min	-.002	2	-.009	3	0	10	-4.953e-4	5	7264.34	2	NC	1
71	17	max	.002	3	.007	2	.033	4	6.24e-5	3	NC	1	NC	1
72		min	-.002	2	-.009	3	0	10	-4.955e-4	5	6261.651	2	NC	1
73	18	max	.002	3	.008	2	.035	4	6.619e-5	3	NC	1	NC	1
74		min	-.003	2	-.009	3	0	10	-4.956e-4	5	5475.275	2	NC	1
75	19	max	.003	3	.009	2	.036	4	6.998e-5	3	NC	3	NC	1
76		min	-.003	2	-.009	3	0	10	-4.958e-4	5	4853.133	2	NC	1
77	M4	1	max	0	.012	2	0	10	2.772e-3	5	NC	1	NC	1
78		min	0	15	-.011	3	-.038	4	-8.119e-5	1	NC	1	511.659	4
79	2	max	0	1	.012	2	0	10	2.772e-3	5	NC	1	NC	1
80		min	0	15	-.01	3	-.035	4	-8.119e-5	1	NC	1	557.666	4
81	3	max	0	1	.011	2	0	10	2.772e-3	5	NC	1	NC	1
82		min	0	15	-.01	3	-.032	4	-8.119e-5	1	NC	1	612.408	4
83	4	max	0	1	.01	2	0	10	2.772e-3	5	NC	1	NC	1
84		min	0	15	-.009	3	-.028	4	-8.119e-5	1	NC	1	678.187	4
85	5	max	0	1	.01	2	0	10	2.772e-3	5	NC	1	NC	1
86		min	0	15	-.009	3	-.025	4	-8.119e-5	1	NC	1	758.136	4
87	6	max	0	1	.009	2	0	10	2.772e-3	5	NC	1	NC	1
88		min	0	15	-.008	3	-.023	4	-8.119e-5	1	NC	1	856.612	4
89	7	max	0	1	.008	2	0	10	2.772e-3	5	NC	1	NC	1
90		min	0	15	-.007	3	-.02	4	-8.119e-5	1	NC	1	979.821	4
91	8	max	0	1	.008	2	0	10	2.772e-3	5	NC	1	NC	1
92		min	0	15	-.007	3	-.017	4	-8.119e-5	1	NC	1	1136.846	4
93	9	max	0	1	.007	2	0	10	2.772e-3	5	NC	1	NC	1
94		min	0	15	-.006	3	-.014	4	-8.119e-5	1	NC	1	1341.426	4
95	10	max	0	1	.006	2	0	10	2.772e-3	5	NC	1	NC	1
96		min	0	15	-.006	3	-.012	4	-8.119e-5	1	NC	1	1615.158	4
97	11	max	0	1	.006	2	0	10	2.772e-3	5	NC	1	NC	1
98		min	0	15	-.005	3	-.01	4	-8.119e-5	1	NC	1	1993.603	4
99	12	max	0	1	.005	2	0	10	2.772e-3	5	NC	1	NC	1





Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.005	4	1.996e-6	9	NC	1	NC	1
158			min	0	2	-.004	3	0	9	-5.032e-4	4	NC	1	NC	1
159		4	max	.001	3	.003	2	.008	4	1.792e-6	9	NC	1	NC	1
160			min	-.001	2	-.006	3	0	9	-4.958e-4	4	NC	1	NC	1
161		5	max	.001	3	.005	2	.01	4	1.588e-6	9	NC	1	NC	1
162			min	-.002	2	-.007	3	0	9	-4.884e-4	4	NC	1	NC	1
163		6	max	.002	3	.006	2	.013	4	2.557e-5	3	NC	1	NC	1
164			min	-.002	2	-.009	3	0	9	-4.809e-4	4	8056.551	2	NC	1
165		7	max	.002	3	.007	2	.015	4	5.392e-5	3	NC	1	NC	1
166			min	-.002	2	-.011	3	0	9	-4.735e-4	4	6666.518	2	NC	1
167		8	max	.002	3	.008	2	.018	4	8.227e-5	3	NC	1	NC	1
168			min	-.003	2	-.012	3	0	9	-4.661e-4	4	5645.726	2	NC	1
169		9	max	.003	3	.009	2	.02	4	1.106e-4	3	NC	1	NC	1
170			min	-.003	2	-.014	3	0	9	-4.586e-4	4	4860.513	2	NC	1
171		10	max	.003	3	.011	2	.022	4	1.39e-4	3	NC	3	NC	1
172			min	-.004	2	-.015	3	0	9	-4.512e-4	4	4236.682	2	NC	1
173		11	max	.004	3	.012	2	.024	4	1.673e-4	3	NC	3	NC	1
174			min	-.004	2	-.017	3	0	9	-4.438e-4	4	3729.533	2	NC	1
175		12	max	.004	3	.014	2	.026	4	1.957e-4	3	NC	3	NC	1
176			min	-.005	2	-.018	3	0	9	-4.364e-4	4	3310.313	2	NC	1
177		13	max	.004	3	.016	2	.028	4	2.24e-4	3	NC	3	NC	1
178			min	-.005	2	-.019	3	0	9	-4.289e-4	4	2959.496	2	NC	1
179		14	max	.005	3	.017	2	.03	4	2.524e-4	3	NC	3	NC	1
180			min	-.005	2	-.02	3	0	9	-4.215e-4	4	2663.215	2	NC	1
181		15	max	.005	3	.019	2	.031	4	2.807e-4	3	NC	3	NC	1
182			min	-.006	2	-.021	3	0	9	-4.141e-4	4	2411.255	2	NC	1
183		16	max	.005	3	.021	2	.033	4	3.091e-4	3	NC	3	NC	1
184			min	-.006	2	-.022	3	0	9	-4.066e-4	4	2195.862	2	NC	1
185		17	max	.006	3	.023	2	.034	4	3.374e-4	3	NC	3	NC	1
186			min	-.007	2	-.023	3	0	9	-3.992e-4	4	2011.016	2	NC	1
187		18	max	.006	3	.025	2	.036	4	3.658e-4	3	NC	3	NC	1
188			min	-.007	2	-.024	3	0	9	-3.918e-4	4	1851.958	2	NC	1
189		19	max	.006	3	.027	2	.037	4	3.941e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	0	9	-3.844e-4	4	1714.881	2	NC	1
191	M8	1	max	.003	2	.035	2	0	9	2.659e-3	4	NC	1	NC	1
192			min	0	15	-.03	3	-.039	4	-2.674e-4	3	NC	1	501.775	4
193		2	max	.002	2	.033	2	0	9	2.659e-3	4	NC	1	NC	1
194			min	0	15	-.028	3	-.035	4	-2.674e-4	3	NC	1	546.896	4
195		3	max	.002	2	.031	2	0	9	2.659e-3	4	NC	1	NC	1
196			min	0	15	-.027	3	-.032	4	-2.674e-4	3	NC	1	600.585	4
197		4	max	.002	2	.029	2	0	9	2.659e-3	4	NC	1	NC	1
198			min	0	15	-.025	3	-.029	4	-2.674e-4	3	NC	1	665.099	4
199		5	max	.002	2	.027	2	0	9	2.659e-3	4	NC	1	NC	1
200			min	0	15	-.023	3	-.026	4	-2.674e-4	3	NC	1	743.511	4
201		6	max	.002	2	.025	2	0	9	2.659e-3	4	NC	1	NC	1
202			min	0	15	-.022	3	-.023	4	-2.674e-4	3	NC	1	840.095	4
203		7	max	.002	2	.023	2	0	9	2.659e-3	4	NC	1	NC	1
204			min	0	15	-.02	3	-.02	4	-2.674e-4	3	NC	1	960.937	4
205		8	max	.002	2	.022	2	0	9	2.659e-3	4	NC	1	NC	1
206			min	0	15	-.018	3	-.017	4	-2.674e-4	3	NC	1	1114.947	4
207		9	max	.001	2	.02	2	0	9	2.659e-3	4	NC	1	NC	1
208			min	0	15	-.017	3	-.015	4	-2.674e-4	3	NC	1	1315.6	4
209		10	max	.001	2	.018	2	0	9	2.659e-3	4	NC	1	NC	1
210			min	0	15	-.015	3	-.012	4	-2.674e-4	3	NC	1	1584.079	4
211		11	max	.001	2	.016	2	0	9	2.659e-3	4	NC	1	NC	1
212			min	0	15	-.013	3	-.01	4	-2.674e-4	3	NC	1	1955.265	4
213		12	max	.001	2	.014	2	0	9	2.659e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214		min	0	15	-.012	3	-.008	4	-2.674e-4	3	NC	1	2489.996	4
215		max	0	2	.012	2	0	9	2.659e-3	4	NC	1	NC	1
216		min	0	15	-.01	3	-.006	4	-2.674e-4	3	NC	1	3302.376	4
217		max	0	2	.01	2	0	9	2.659e-3	4	NC	1	NC	1
218		min	0	15	-.008	3	-.004	4	-2.674e-4	3	NC	1	4627.743	4
219		max	0	2	.008	2	0	9	2.659e-3	4	NC	1	NC	1
220		min	0	15	-.007	3	-.003	4	-2.674e-4	3	NC	1	7018.931	4
221		max	0	2	.006	2	0	9	2.659e-3	4	NC	1	NC	1
222		min	0	15	-.005	3	-.002	4	-2.674e-4	3	NC	1	NC	1
223		max	0	2	.004	2	0	9	2.659e-3	4	NC	1	NC	1
224		min	0	15	-.003	3	0	4	-2.674e-4	3	NC	1	NC	1
225		max	0	2	.002	2	0	9	2.659e-3	4	NC	1	NC	1
226		min	0	15	-.002	3	0	4	-2.674e-4	3	NC	1	NC	1
227		max	0	1	0	1	0	1	2.659e-3	4	NC	1	NC	1
228		min	0	1	0	1	0	1	-2.674e-4	3	NC	1	NC	1
229	M10	max	.002	2	.011	2	0	10	1.191e-4	1	NC	3	NC	1
230		min	-.002	3	-.011	3	-.006	4	-6.375e-4	3	3927.767	2	NC	1
231		max	.002	2	.01	2	0	10	1.288e-4	14	NC	3	NC	1
232		min	-.002	3	-.011	3	-.006	4	-6.141e-4	3	4296.066	2	NC	1
233		max	.002	2	.009	2	0	10	1.726e-4	4	NC	3	NC	1
234		min	-.002	3	-.01	3	-.006	4	-5.908e-4	3	4735.801	2	NC	1
235		max	.002	2	.008	2	0	10	2.187e-4	4	NC	1	NC	1
236		min	-.002	3	-.01	3	-.006	4	-5.674e-4	3	5264.443	2	NC	1
237		max	.002	2	.007	2	0	10	2.649e-4	4	NC	1	NC	1
238		min	-.002	3	-.009	3	-.006	4	-5.44e-4	3	5905.35	2	NC	1
239		max	.001	2	.006	2	0	10	3.11e-4	4	NC	1	NC	1
240		min	-.002	3	-.009	3	-.006	4	-5.206e-4	3	6690.239	2	NC	1
241		max	.001	2	.006	2	0	3	3.572e-4	4	NC	1	NC	1
242		min	-.002	3	-.008	3	-.006	4	-4.972e-4	3	7663.006	2	NC	1
243		max	.001	2	.005	2	0	3	4.033e-4	4	NC	1	NC	1
244		min	-.001	3	-.008	3	-.006	4	-4.738e-4	3	8885.744	2	NC	1
245		max	.001	2	.004	2	0	3	4.494e-4	4	NC	1	NC	1
246		min	-.001	3	-.007	3	-.006	4	-4.504e-4	3	NC	1	NC	1
247		max	0	2	.003	2	0	3	4.956e-4	4	NC	1	NC	1
248		min	-.001	3	-.007	3	-.005	4	-4.27e-4	3	NC	1	NC	1
249		max	0	2	.003	2	0	3	5.417e-4	4	NC	1	NC	1
250		min	-.001	3	-.006	3	-.005	4	-4.036e-4	3	NC	1	NC	1
251		max	0	2	.002	2	0	3	5.879e-4	4	NC	1	NC	1
252		min	0	3	-.005	3	-.004	4	-3.802e-4	3	NC	1	NC	1
253		max	0	2	.002	2	0	3	6.34e-4	4	NC	1	NC	1
254		min	0	3	-.005	3	-.004	4	-3.568e-4	3	NC	1	NC	1
255		max	0	2	.001	2	0	3	6.802e-4	4	NC	1	NC	1
256		min	0	3	-.004	3	-.003	4	-3.334e-4	3	NC	1	NC	1
257		max	0	2	0	2	0	3	7.263e-4	4	NC	1	NC	1
258		min	0	3	-.003	3	-.003	4	-3.1e-4	3	NC	1	NC	1
259		max	0	2	0	2	0	3	7.725e-4	4	NC	1	NC	1
260		min	0	3	-.002	3	-.002	4	-2.866e-4	3	NC	1	NC	1
261		max	0	2	0	2	0	3	8.186e-4	4	NC	1	NC	1
262		min	0	3	-.002	3	-.001	4	-2.632e-4	3	NC	1	NC	1
263		max	0	2	0	2	0	3	8.647e-4	4	NC	1	NC	1
264		min	0	3	0	3	0	4	-2.398e-4	3	NC	1	NC	1
265		max	0	1	0	1	0	1	9.109e-4	4	NC	1	NC	1
266		min	0	1	0	1	0	1	-2.164e-4	3	NC	1	NC	1
267	M11	max	0	1	0	1	0	1	1.036e-4	3	NC	1	NC	1
268		min	0	1	0	1	0	1	-4.354e-4	4	NC	1	NC	1
269		max	0	3	0	2	.002	4	7.74e-5	3	NC	1	NC	1
270		min	0	2	0	3	0	3	-4.6e-4	4	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271	3	max	0	3	0	2	.004	4	5.116e-5	3	NC	1	NC	1
272		min	0	2	-.002	3	0	3	-4.846e-4	4	NC	1	NC	1
273	4	max	0	3	0	2	.007	4	2.492e-5	3	NC	1	NC	1
274		min	0	2	-.003	3	-.001	3	-5.092e-4	4	NC	1	NC	1
275	5	max	0	3	0	2	.009	4	7.793e-6	10	NC	1	NC	1
276		min	0	2	-.004	3	-.002	3	-5.339e-4	4	NC	1	NC	1
277	6	max	0	3	0	2	.011	4	8.994e-6	10	NC	1	NC	1
278		min	0	2	-.004	3	-.002	3	-5.585e-4	4	NC	1	NC	1
279	7	max	0	3	0	2	.013	4	1.019e-5	10	NC	1	NC	1
280		min	0	2	-.005	3	-.002	3	-5.831e-4	4	NC	1	NC	1
281	8	max	.001	3	.001	2	.015	4	1.139e-5	10	NC	1	NC	1
282		min	-.001	2	-.006	3	-.002	3	-6.077e-4	4	NC	1	NC	1
283	9	max	.001	3	.001	2	.017	5	1.259e-5	10	NC	1	NC	1
284		min	-.001	2	-.007	3	-.003	3	-6.323e-4	4	NC	1	NC	1
285	10	max	.001	3	.002	2	.019	5	1.379e-5	10	NC	1	NC	1
286		min	-.001	2	-.007	3	-.003	3	-6.57e-4	4	NC	1	NC	1
287	11	max	.001	3	.002	2	.021	5	1.499e-5	10	NC	1	NC	1
288		min	-.002	2	-.008	3	-.003	3	-6.816e-4	4	NC	1	NC	1
289	12	max	.002	3	.003	2	.023	5	1.619e-5	10	NC	1	NC	1
290		min	-.002	2	-.008	3	-.003	3	-7.062e-4	4	NC	1	NC	1
291	13	max	.002	3	.004	2	.024	5	1.739e-5	10	NC	1	NC	1
292		min	-.002	2	-.008	3	-.003	3	-7.308e-4	4	NC	1	NC	1
293	14	max	.002	3	.004	2	.026	5	1.859e-5	10	NC	1	NC	1
294		min	-.002	2	-.009	3	-.003	3	-7.554e-4	4	NC	1	NC	1
295	15	max	.002	3	.005	2	.027	5	1.98e-5	10	NC	1	NC	1
296		min	-.002	2	-.009	3	-.003	3	-7.8e-4	4	8567.459	2	NC	1
297	16	max	.002	3	.006	2	.029	5	2.1e-5	10	NC	1	NC	1
298		min	-.002	2	-.009	3	-.003	3	-8.047e-4	4	7272.275	2	NC	1
299	17	max	.002	3	.007	2	.03	5	2.22e-5	10	NC	1	NC	1
300		min	-.002	2	-.009	3	-.003	3	-8.293e-4	4	6267.934	2	NC	1
301	18	max	.002	3	.008	2	.032	5	2.34e-5	10	NC	1	NC	1
302		min	-.003	2	-.009	3	-.002	3	-8.539e-4	4	5480.376	2	NC	1
303	19	max	.003	3	.009	2	.033	5	2.46e-5	10	NC	3	NC	1
304		min	-.003	2	-.009	3	-.002	3	-8.785e-4	4	4857.373	2	NC	1
305	M12	1	max	0	.012	2	.001	3	3.116e-3	4	NC	1	NC	1
306		min	0	5	-.011	3	-.036	5	-3.082e-5	10	NC	1	541.234	5
307	2	max	0	1	.012	2	.001	3	3.116e-3	4	NC	1	NC	1
308		min	0	5	-.011	3	-.033	5	-3.082e-5	10	NC	1	589.885	5
309	3	max	0	1	.011	2	.001	3	3.116e-3	4	NC	1	NC	1
310		min	0	5	-.01	3	-.03	5	-3.082e-5	10	NC	1	647.773	5
311	4	max	0	1	.01	2	0	3	3.116e-3	4	NC	1	NC	1
312		min	0	5	-.009	3	-.027	5	-3.082e-5	10	NC	1	717.33	5
313	5	max	0	1	.01	2	0	3	3.116e-3	4	NC	1	NC	1
314		min	0	5	-.009	3	-.024	5	-3.082e-5	10	NC	1	801.868	5
315	6	max	0	1	.009	2	0	3	3.116e-3	4	NC	1	NC	1
316		min	0	5	-.008	3	-.021	5	-3.082e-5	10	NC	1	905.995	5
317	7	max	0	1	.008	2	0	3	3.116e-3	4	NC	1	NC	1
318		min	0	5	-.007	3	-.019	5	-3.082e-5	10	NC	1	1036.271	5
319	8	max	0	1	.008	2	0	3	3.116e-3	4	NC	1	NC	1
320		min	0	5	-.007	3	-.016	5	-3.082e-5	10	NC	1	1202.3	5
321	9	max	0	1	.007	2	0	3	3.116e-3	4	NC	1	NC	1
322		min	0	5	-.006	3	-.014	5	-3.082e-5	10	NC	1	1418.607	5
323	10	max	0	1	.006	2	0	3	3.116e-3	4	NC	1	NC	1
324		min	0	5	-.006	3	-.011	5	-3.082e-5	10	NC	1	1708.022	5
325	11	max	0	1	.006	2	0	3	3.116e-3	4	NC	1	NC	1
326		min	0	5	-.005	3	-.009	5	-3.082e-5	10	NC	1	2108.143	5
327	12	max	0	1	.005	2	0	3	3.116e-3	4	NC	1	NC	1



Company : Schletter, Inc.
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	5	-.004	3	-.007	5	-3.082e-5	10	NC	1	2684.541	5
329		13	max	0	1	.004	2	0	3	3.116e-3	4	NC	1	NC	1
330			min	0	5	-.004	3	-.005	5	-3.082e-5	10	NC	1	3560.199	5
331		14	max	0	1	.003	2	0	3	3.116e-3	4	NC	1	NC	1
332			min	0	5	-.003	3	-.004	5	-3.082e-5	10	NC	1	4988.763	5
333		15	max	0	1	.003	2	0	3	3.116e-3	4	NC	1	NC	1
334			min	0	5	-.002	3	-.003	5	-3.082e-5	10	NC	1	7566.06	5
335		16	max	0	1	.002	2	0	3	3.116e-3	4	NC	1	NC	1
336			min	0	5	-.002	3	-.001	5	-3.082e-5	10	NC	1	NC	1
337		17	max	0	1	.001	2	0	3	3.116e-3	4	NC	1	NC	1
338			min	0	5	-.001	3	0	5	-3.082e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	3	3.116e-3	4	NC	1	NC	1
340			min	0	5	0	3	0	5	-3.082e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	3.116e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-3.082e-5	10	NC	1	NC	1
343	M1	1	max	.01	3	.027	3	.006	5	3.921e-3	2	NC	1	NC	1
344			min	-.01	2	-.021	2	0	9	-5.817e-3	3	NC	1	NC	1
345		2	max	.01	3	.016	3	.007	5	1.951e-3	2	NC	4	NC	1
346			min	-.01	2	-.013	2	0	9	-2.859e-3	3	5558.076	2	NC	1
347		3	max	.01	3	.007	3	.009	5	2.38e-4	5	NC	4	NC	1
348			min	-.01	2	-.005	2	0	9	-5.761e-5	9	2818.391	3	NC	1
349		4	max	.01	3	.002	2	.011	5	2.383e-4	5	NC	4	NC	1
350			min	-.01	2	-.002	3	-.001	9	-4.871e-5	9	1888.413	3	NC	1
351		5	max	.01	3	.009	2	.012	5	2.386e-4	5	NC	4	NC	1
352			min	-.01	2	-.008	3	-.001	9	-3.982e-5	9	1478.701	3	7580.438	5
353		6	max	.01	3	.014	2	.014	5	2.389e-4	5	NC	4	NC	1
354			min	-.01	2	-.014	3	-.001	9	-3.093e-5	9	1257.464	3	5749.767	5
355		7	max	.01	3	.018	2	.016	5	2.392e-4	5	NC	4	NC	1
356			min	-.01	2	-.018	3	0	9	-2.203e-5	9	1127.604	3	4582.417	5
357		8	max	.01	3	.021	2	.019	5	2.395e-4	5	NC	4	NC	1
358			min	-.01	2	-.021	3	0	9	-1.314e-5	9	1051.15	3	3784.622	5
359		9	max	.009	3	.024	2	.021	5	2.403e-4	4	NC	4	NC	1
360			min	-.01	2	-.023	3	0	9	-4.243e-6	9	1011.037	3	3211.971	5
361		10	max	.009	3	.025	2	.023	4	2.438e-4	4	NC	4	NC	1
362			min	-.01	2	-.024	3	0	9	-2.607e-6	10	999.601	3	2775.223	4
363		11	max	.009	3	.024	2	.025	4	2.472e-4	4	NC	4	NC	1
364			min	-.01	2	-.023	3	0	10	-6.127e-6	10	1014.586	3	2436.459	4
365		12	max	.009	3	.023	2	.028	4	2.507e-4	4	NC	4	NC	1
366			min	-.01	2	-.021	3	0	10	-9.648e-6	10	1058.087	3	2174.841	4
367		13	max	.009	3	.02	2	.03	4	2.542e-4	4	NC	4	NC	1
368			min	-.01	2	-.018	3	0	10	-1.317e-5	10	1137.416	3	1969.458	4
369		14	max	.009	3	.015	2	.032	4	2.576e-4	4	NC	4	NC	1
370			min	-.01	2	-.014	3	0	10	-1.669e-5	10	1268.658	3	1806.338	4
371		15	max	.009	3	.009	2	.034	4	2.611e-4	4	NC	4	NC	1
372			min	-.01	2	-.008	3	0	10	-2.021e-5	10	1486.735	3	1675.882	4
373		16	max	.009	3	.002	2	.036	4	4.047e-4	4	NC	4	NC	1
374			min	-.01	2	-.002	3	0	10	-2.279e-5	10	1877.52	3	1571.332	4
375		17	max	.009	3	.006	3	.038	4	3.885e-3	4	NC	4	NC	1
376			min	-.01	2	-.008	2	0	10	-2.862e-6	10	2724.254	3	1488.005	4
377		18	max	.009	3	.014	3	.039	4	2.894e-3	2	NC	1	NC	1
378			min	-.01	2	-.018	2	0	10	-1.711e-3	3	5341.638	3	1422.12	4
379		19	max	.009	3	.023	3	.04	4	5.843e-3	2	NC	1	NC	1
380			min	-.01	2	-.029	2	0	9	-3.572e-3	3	5736.633	2	1372.78	4
381	M5	1	max	.026	3	.076	3	.006	5	2.777e-5	4	NC	1	NC	1
382			min	-.027	2	-.061	2	0	9	7.854e-8	9	4204.935	3	NC	1
383		2	max	.026	3	.046	3	.007	5	1.757e-4	3	NC	4	NC	1
384			min	-.027	2	-.037	2	0	9	-2.069e-6	9	1914.723	2	NC	1



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Job Number :
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.026	3	.018	3	.009	5	3.263e-4	3	NC	4	NC	1
386		min	-.027	2	-.014	2	0	9	-4.172e-6	9	981.355	2	NC	1
387	4	max	.026	3	.007	2	.011	4	3.128e-4	3	NC	5	NC	1
388		min	-.027	2	-.006	3	0	9	-3.901e-6	9	664.131	3	9782.011	3
389	5	max	.026	3	.025	2	.013	4	2.992e-4	3	NC	5	NC	1
390		min	-.027	2	-.026	3	0	9	-3.629e-6	9	519.191	3	8198.947	3
391	6	max	.026	3	.041	2	.015	4	2.856e-4	3	NC	5	NC	1
392		min	-.027	2	-.041	3	0	9	-3.358e-6	9	441.597	3	7422.814	3
393	7	max	.025	3	.053	2	.017	4	2.72e-4	3	NC	5	NC	1
394		min	-.027	2	-.054	3	0	9	-3.087e-6	9	396.404	3	7080.451	3
395	8	max	.025	3	.063	2	.02	4	2.585e-4	3	NC	5	NC	1
396		min	-.027	2	-.062	3	0	9	-2.815e-6	9	370.081	3	7030.076	3
397	9	max	.025	3	.069	2	.022	4	2.515e-4	4	NC	5	NC	1
398		min	-.027	2	-.067	3	0	9	-2.544e-6	9	356.587	3	7220.608	3
399	10	max	.025	3	.071	2	.024	4	2.59e-4	4	NC	5	NC	1
400		min	-.027	2	-.068	3	0	9	-2.272e-6	9	352.662	2	7650.516	3
401	11	max	.025	3	.071	2	.027	4	2.664e-4	4	NC	5	NC	1
402		min	-.027	2	-.066	3	0	9	-2.001e-6	9	355.116	2	8358.72	3
403	12	max	.025	3	.066	2	.029	4	2.738e-4	4	NC	5	NC	1
404		min	-.027	2	-.06	3	0	9	-1.729e-6	9	368.338	2	9433.487	3
405	13	max	.025	3	.057	2	.031	4	2.813e-4	4	NC	5	NC	1
406		min	-.027	2	-.051	3	0	9	-1.458e-6	9	395.509	2	NC	1
407	14	max	.025	3	.044	2	.034	4	2.887e-4	4	NC	5	NC	1
408		min	-.027	2	-.039	3	0	9	-1.186e-6	9	443.947	2	NC	1
409	15	max	.025	3	.027	2	.035	4	2.961e-4	4	NC	5	NC	1
410		min	-.027	2	-.024	3	0	9	-9.147e-7	9	530.603	3	NC	1
411	16	max	.025	3	.005	2	.037	4	4.418e-4	4	NC	5	NC	1
412		min	-.027	2	-.005	3	0	9	-9.001e-7	9	670.97	3	NC	1
413	17	max	.025	3	.017	3	.038	4	3.877e-3	4	NC	4	NC	1
414		min	-.027	2	-.022	2	0	9	-7.001e-6	9	973.179	3	NC	1
415	18	max	.025	3	.04	3	.04	4	1.993e-3	4	NC	4	NC	1
416		min	-.027	2	-.053	2	0	9	-3.658e-6	9	1908.322	3	NC	1
417	19	max	.025	3	.065	3	.04	4	8.07e-6	5	NC	3	NC	1
418		min	-.027	2	-.085	2	0	9	-3.933e-6	3	1967.104	2	NC	1
419	M9	1	max	.01	.025	3	.006	5	5.861e-3	3	NC	1	NC	1
420		min	-.01	2	-.021	2	0	9	-3.921e-3	2	NC	1	NC	1
421	2	max	.01	3	.015	3	.006	4	2.867e-3	3	NC	4	NC	1
422		min	-.01	2	-.013	2	0	10	-1.951e-3	2	4617.154	3	NC	1
423	3	max	.01	3	.005	3	.006	4	5.554e-5	1	NC	4	NC	1
424		min	-.01	2	-.005	2	0	10	-7.081e-5	3	2388.287	3	NC	1
425	4	max	.01	3	.002	2	.006	4	4.637e-5	1	NC	4	NC	1
426		min	-.01	2	-.003	3	0	3	-7.398e-5	3	1696.708	3	NC	1
427	5	max	.01	3	.009	2	.007	4	3.72e-5	1	NC	4	NC	1
428		min	-.01	2	-.01	3	-.002	3	-7.715e-5	3	1365.979	3	8252.455	3
429	6	max	.01	3	.014	2	.008	4	2.803e-5	1	NC	4	NC	1
430		min	-.01	2	-.015	3	-.003	3	-8.032e-5	3	1180.544	3	7150.266	3
431	7	max	.01	3	.018	2	.01	4	1.886e-5	1	NC	4	NC	1
432		min	-.01	2	-.019	3	-.004	3	-8.348e-5	3	1069.876	3	6509.039	3
433	8	max	.01	3	.021	2	.012	4	1.03e-5	4	NC	4	NC	1
434		min	-.01	2	-.022	3	-.004	3	-8.665e-5	3	1004.775	3	6146.85	3
435	9	max	.01	3	.024	2	.014	4	1.755e-5	4	NC	4	NC	1
436		min	-.01	2	-.024	3	-.005	3	-8.982e-5	3	971.76	3	5642.162	4
437	10	max	.01	3	.025	2	.017	4	2.722e-5	5	NC	4	NC	1
438		min	-.01	2	-.024	3	-.005	3	-9.299e-5	3	964.83	3	4374.471	4
439	11	max	.01	3	.024	2	.019	5	3.734e-5	5	NC	4	NC	1
440		min	-.01	2	-.023	3	-.005	3	-9.615e-5	3	982.558	3	3528.512	5
441	12	max	.009	3	.023	2	.022	5	4.746e-5	5	NC	4	NC	1





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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	0	9	.05	3	.014	3	6.062e-3	2	NC	4	NC	1
500		min	-.04	4	-.072	2	-.011	2	-4.709e-3	3	1684.602	2	NC	1
501	4	max	0	9	.06	3	.017	3	6.963e-3	2	NC	4	NC	1
502		min	-.04	4	-.087	2	-.013	2	-5.372e-3	3	1242.198	2	9269.287	3
503	5	max	0	9	.066	3	.02	3	7.865e-3	2	NC	4	NC	1
504		min	-.04	4	-.097	2	-.015	2	-6.035e-3	3	1068.299	2	6939.224	3
505	6	max	0	9	.07	3	.022	3	8.766e-3	2	NC	4	NC	1
506		min	-.04	4	-.1	2	-.018	2	-6.698e-3	3	1015.127	2	5689.133	3
507	7	max	0	9	.07	3	.024	3	9.668e-3	2	NC	4	NC	1
508		min	-.04	4	-.099	2	-.021	2	-7.361e-3	3	1038.173	2	4987.565	3
509	8	max	0	9	.068	3	.025	3	1.057e-2	2	NC	4	NC	1
510		min	-.04	4	-.094	2	-.024	2	-8.024e-3	3	1118.157	2	4625.513	3
511	9	max	0	9	.066	3	.025	3	1.147e-2	2	NC	4	NC	4
512		min	-.04	4	-.088	2	-.026	2	-8.687e-3	3	1226.981	2	4329.003	2
513	10	max	0	9	.065	3	.025	3	1.237e-2	2	NC	4	NC	4
514		min	-.04	4	-.085	2	-.027	2	-9.35e-3	3	1290.92	2	4098.569	2
515	11	max	0	9	.066	3	.023	3	1.147e-2	2	NC	4	NC	4
516		min	-.04	4	-.088	2	-.026	2	-8.682e-3	3	1226.981	2	4329.007	2
517	12	max	0	9	.068	3	.022	3	1.057e-2	2	NC	4	NC	1
518		min	-.04	4	-.094	2	-.024	2	-8.015e-3	3	1118.157	2	5013.301	2
519	13	max	0	9	.07	3	.02	3	9.668e-3	2	NC	4	NC	1
520		min	-.04	4	-.099	2	-.021	2	-7.347e-3	3	1038.173	2	6306.074	2
521	14	max	0	9	.07	3	.018	3	8.767e-3	2	NC	4	NC	1
522		min	-.04	4	-.1	2	-.018	2	-6.679e-3	3	1015.127	2	8198.594	3
523	15	max	0	9	.066	3	.016	3	7.866e-3	2	NC	4	NC	1
524		min	-.04	4	-.097	2	-.015	2	-6.012e-3	3	1068.299	2	NC	1
525	16	max	0	9	.06	3	.014	3	6.964e-3	2	NC	4	NC	1
526		min	-.04	4	-.087	2	-.013	2	-5.344e-3	3	1242.198	2	NC	1
527	17	max	0	9	.05	3	.012	3	6.063e-3	2	NC	4	NC	1
528		min	-.04	4	-.072	2	-.011	2	-4.677e-3	3	1684.602	2	NC	1
529	18	max	0	9	.037	3	.01	3	5.162e-3	2	NC	4	NC	1
530		min	-.04	4	-.052	2	-.01	2	-4.009e-3	3	3162.337	2	NC	1
531	19	max	0	9	.023	3	.009	3	4.26e-3	2	NC	1	NC	1
532		min	-.04	4	-.029	2	-.01	2	-3.342e-3	3	NC	1	NC	1
533	M15	max	0	1	0	1	0	1	3.968e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.268e-4	5	NC	1	NC	1
535	2	max	0	3	0	5	.003	4	7.052e-4	3	NC	1	NC	1
536		min	0	4	-.001	3	0	3	-6.26e-4	5	NC	1	NC	1
537	3	max	0	3	.002	5	.007	4	1.014e-3	3	NC	1	NC	1
538		min	0	4	-.002	3	-.003	3	-6.322e-4	2	NC	1	7663.338	4
539	4	max	0	3	.002	5	.012	4	1.322e-3	3	NC	1	NC	9
540		min	-.001	4	-.003	3	-.006	3	-9.324e-4	2	NC	1	4703.056	4
541	5	max	0	3	.003	5	.017	4	1.63e-3	3	NC	1	NC	9
542		min	-.001	4	-.004	3	-.01	3	-1.233e-3	2	NC	1	3155.807	3
543	6	max	0	3	.004	5	.021	4	1.939e-3	3	NC	1	NC	9
544		min	-.002	4	-.005	3	-.015	3	-1.533e-3	2	NC	1	2299.466	3
545	7	max	0	3	.004	5	.025	4	2.247e-3	3	NC	2	NC	9
546		min	-.002	4	-.005	3	-.02	3	-1.833e-3	2	9377.21	1	1798.49	3
547	8	max	0	3	.005	5	.027	4	2.556e-3	3	NC	2	9102.395	9
548		min	-.002	4	-.006	3	-.025	3	-2.133e-3	2	8658.97	1	1483.46	3
549	9	max	.001	3	.005	5	.029	4	2.864e-3	3	NC	2	7977.57	9
550		min	-.003	4	-.006	3	-.029	3	-2.434e-3	2	8272.37	1	1277.263	3
551	10	max	.001	3	.006	5	.029	4	3.172e-3	3	NC	2	7232.309	9
552		min	-.003	4	-.006	3	-.033	3	-2.734e-3	2	8150.069	1	1141.157	3
553	11	max	.001	3	.006	5	.028	4	3.481e-3	3	NC	2	6767.212	9
554		min	-.004	4	-.006	3	-.035	3	-3.034e-3	2	8272.37	1	1054.801	3
555	12	max	.002	3	.006	5	.027	2	3.789e-3	3	NC	2	6535.115	9



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00

Maximum concrete compression stress (psi): 0

Resultant tension force (lb): 1465

Resultant compression force (lb): 0

Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00

Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00

Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00

Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.