

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	130 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 26.53 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.890 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>7%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.310 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>8%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

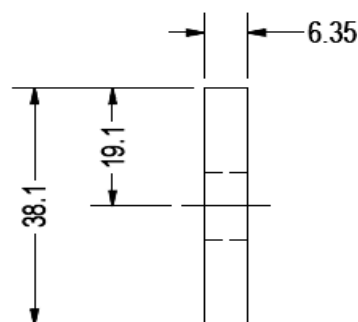
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.661 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	<b>10%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.002 k-ft
$P_n$ =	0.153 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>6%</b>



A cross brace kit is required every 35 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

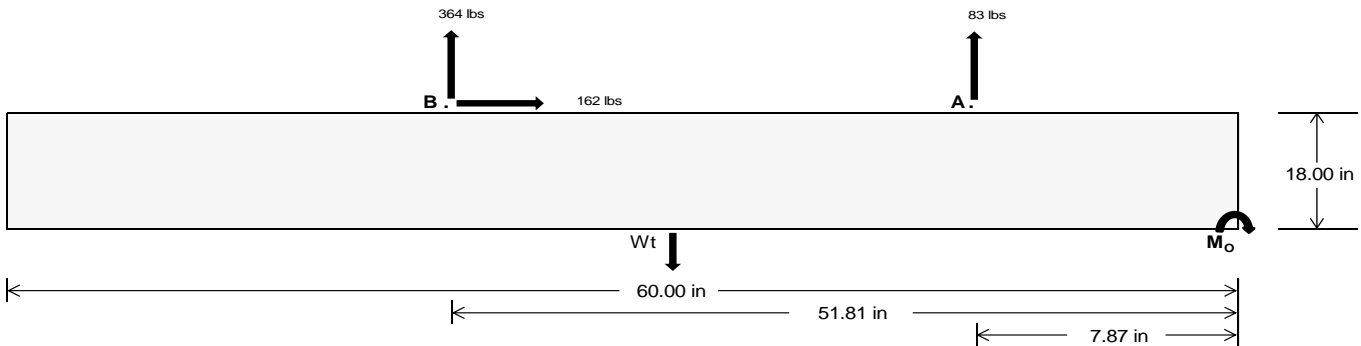
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<b>348.48</b>	<b>1516.11</b>	k
Compressive Load =	<b>1156.55</b>	<b>973.17</b>	k
Lateral Load =	<b>18.73</b>	<b>675.17</b>	k
Moment (Weak Axis) =	<b>0.03</b>	<b>0.00</b>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 22437.6$  in-lbs  
Resisting Force Required = 747.92 lbs  
S.F. = 1.67  
Weight Required = 1246.53 lbs  
Minimum Width = 21 in  
Weight Provided = 1903.13 lbs

### Sliding

Force = 162.30 lbs  
Friction = 0.4  
Weight Required = 405.75 lbs  
Resisting Weight = 1903.13 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 162.30 lbs  
Cohesion = 130 psf  
Area = 8.75 ft<sup>2</sup>  
Resisting = 951.56 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 21in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 21in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

$P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.75 \text{ ft}) =$

Ballast Width			
21 in	22 in	23 in	24 in
1903 lbs	1994 lbs	2084 lbs	2175 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in	21 in	22 in	23 in	24 in
$F_A$	338 lbs	338 lbs	338 lbs	338 lbs	480 lbs	480 lbs	480 lbs	480 lbs	587 lbs	587 lbs	587 lbs	587 lbs	-166 lbs	-166 lbs	-166 lbs	-166 lbs
$F_B$	238 lbs	238 lbs	238 lbs	238 lbs	415 lbs	415 lbs	415 lbs	415 lbs	471 lbs	471 lbs	471 lbs	471 lbs	-728 lbs	-728 lbs	-728 lbs	-728 lbs
$F_V$	19 lbs	19 lbs	19 lbs	19 lbs	284 lbs	284 lbs	284 lbs	284 lbs	226 lbs	226 lbs	226 lbs	226 lbs	-325 lbs	-325 lbs	-325 lbs	-325 lbs
$P_{total}$	2479 lbs	2570 lbs	2660 lbs	2751 lbs	2798 lbs	2889 lbs	2979 lbs	3070 lbs	2961 lbs	3052 lbs	3142 lbs	3233 lbs	248 lbs	302 lbs	356 lbs	411 lbs
$M$	219 lbs-ft	219 lbs-ft	219 lbs-ft	219 lbs-ft	556 lbs-ft	556 lbs-ft	556 lbs-ft	556 lbs-ft	567 lbs-ft	567 lbs-ft	567 lbs-ft	567 lbs-ft	530 lbs-ft	530 lbs-ft	530 lbs-ft	530 lbs-ft
$e$	0.09 ft	0.09 ft	0.08 ft	0.08 ft	0.20 ft	0.19 ft	0.19 ft	0.18 ft	0.19 ft	0.19 ft	0.18 ft	0.18 ft	2.14 ft	1.76 ft	1.49 ft	1.29 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	253.3 psf	251.6 psf	250.2 psf	248.8 psf	243.5 psf	242.3 psf	241.3 psf	240.3 psf	260.7 psf	258.8 psf	257.0 psf	255.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	313.4 psf	309.0 psf	305.1 psf	301.4 psf	396.0 psf	387.9 psf	380.5 psf	373.7 psf	416.1 psf	407.1 psf	398.8 psf	391.3 psf	262.7 psf	147.5 psf	122.4 psf	113.2 psf

Maximum Bearing Pressure = 416 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

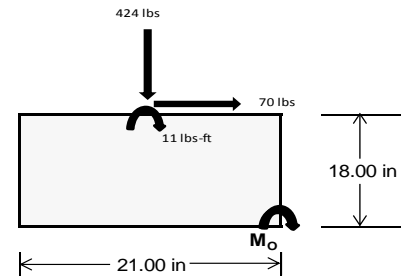
### Overturning Check

$M_o = 256.1 \text{ ft-lbs}$   
 Resisting Force Required = 292.63 lbs  
 S.F. = 1.67  
 Weight Required = 487.72 lbs  
 Minimum Width = 21 in  
 Weight Provided = 1903.13 lbs

*A minimum 60in long x 21in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	21 in			21 in			21 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	104 lbs	50 lbs	48 lbs	207 lbs	424 lbs	165 lbs	71 lbs	-29 lbs	17 lbs
$F_v$	11 lbs	93 lbs	11 lbs	8 lbs	70 lbs	8 lbs	11 lbs	93 lbs	11 lbs
$P_{total}$	2460 lbs	2407 lbs	2404 lbs	2450 lbs	2667 lbs	2408 lbs	760 lbs	660 lbs	706 lbs
$M$	31 lbs-ft	154 lbs-ft	31 lbs-ft	22 lbs-ft	115 lbs-ft	24 lbs-ft	31 lbs-ft	154 lbs-ft	31 lbs-ft
$e$	0.01 ft	0.06 ft	0.01 ft	0.01 ft	0.04 ft	0.01 ft	0.04 ft	0.23 ft	0.04 ft
$L/6$	0.29 ft	1.62 ft	1.72 ft	1.73 ft	1.66 ft	1.73 ft	1.67 ft	1.28 ft	1.66 ft
$f_{min}$	269.2 sqft	214.8 sqft	262.5 sqft	271.4 sqft	259.7 sqft	266.0 sqft	74.8 sqft	15.1 sqft	68.4 sqft
$f_{max}$	293.1 psf	335.3 psf	287.1 psf	288.6 psf	350.0 psf	284.4 psf	98.9 psf	135.7 psf	93.0 psf



Maximum Bearing Pressure = 350 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 21in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 21in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.850 k
Allowable Uplift =	1.214 k
Utilization =	<u>70%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.062 k
Allowable Uplift =	1.116 k
Utilization =	<u>95%</u>



### 6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	0.890 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>16%</u>

#### Diagonal Strut

Maximum Axial Load =	0.310 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>5%</u>



#### Rear Strut

Maximum Axial Load =	1.063 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>19%</u>

#### Bracing

Maximum Axial Load =	0.153 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	29.57 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.591 in
Max Drift, $\Delta_{MAX}$ =	0.046 in
	<u>0.046 ≤ 0.591. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$109.366$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 30.0 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 42.00 \text{ in}$$

$$J = 0.255$$

$$113.57$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.9$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 30.0 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.276 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.37 \\
 &21.005 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.37 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

#### 3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.464 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.





RISA-3D Version 13.0.0    \...\PVMini 60 Cell 1V 20° 130mph 30psf 3.5ft 7-05.r    Page 21



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29	15	max	215.088	1	.083	2	.069	1	0	10	0	4	0	15
30		min	-352.473	3	.013	12	-.485	5	0	4	0	3	0	6
31	16	max	215.195	1	.051	2	.069	1	0	10	0	4	0	15
32		min	-352.393	3	-.005	3	-.582	5	0	4	0	3	0	6
33	17	max	215.302	1	.018	2	.069	1	0	10	0	4	0	15
34		min	-352.313	3	-.03	3	-.678	5	0	4	0	3	0	6
35	18	max	215.408	1	-.014	2	.069	1	0	10	0	14	0	15
36		min	-352.233	3	-.057	4	-.775	5	0	4	0	3	0	6
37	19	max	215.515	1	-.025	15	.069	1	0	10	0	9	0	15
38		min	-352.153	3	-.099	4	-.871	5	0	4	0	3	0	6
39	M3	1	max	103.894	2	1.796	.012	10	0	5	0	4	0	6
40		min	-88.832	3	.421	15	-1.333	4	0	1	0	10	0	15
41	2	max	103.826	2	1.618	6	.012	10	0	5	0	1	0	6
42		min	-88.883	3	.379	15	-1.199	4	0	1	0	10	0	15
43	3	max	103.758	2	1.441	6	.012	10	0	5	0	1	0	2
44		min	-88.934	3	.337	15	-1.066	4	0	1	0	5	0	3
45	4	max	103.691	2	1.263	6	.012	10	0	5	0	1	0	15
46		min	-88.985	3	.295	15	-.932	4	0	1	0	5	0	4
47	5	max	103.623	2	1.085	6	.012	10	0	5	0	1	0	15
48		min	-89.036	3	.254	15	-.799	4	0	1	0	5	0	4
49	6	max	103.555	2	.908	6	.012	10	0	5	0	1	0	15
50		min	-89.087	3	.212	15	-.665	4	0	1	0	5	0	4
51	7	max	103.487	2	.73	6	.012	10	0	5	0	1	0	15
52		min	-89.138	3	.17	15	-.531	4	0	1	0	5	0	4
53	8	max	103.419	2	.552	6	.012	10	0	5	0	1	0	15
54		min	-89.188	3	.128	15	-.398	4	0	1	0	5	0	4
55	9	max	103.351	2	.375	6	.012	10	0	5	0	1	0	15
56		min	-89.239	3	.087	15	-.264	4	0	1	0	5	-.001	4
57	10	max	103.283	2	.197	6	.012	10	0	5	0	1	0	15
58		min	-89.29	3	.045	15	-.13	4	0	1	0	5	-.001	4
59	11	max	103.216	2	.037	2	.027	5	0	5	0	1	0	15
60		min	-89.341	3	-.003	3	-.102	1	0	1	0	5	-.001	4
61	12	max	103.148	2	-.039	15	.161	5	0	5	0	1	0	15
62		min	-89.392	3	-.158	4	-.102	1	0	1	0	5	-.001	4
63	13	max	103.08	2	-.08	15	.294	5	0	5	0	1	0	15
64		min	-89.443	3	-.336	4	-.102	1	0	1	0	5	-.001	4
65	14	max	103.012	2	-.122	15	.428	5	0	5	0	9	0	15
66		min	-89.494	3	-.514	4	-.102	1	0	1	0	5	-.001	4
67	15	max	102.944	2	-.164	15	.561	5	0	5	0	10	0	15
68		min	-89.545	3	-.691	4	-.102	1	0	1	0	4	0	4
69	16	max	102.876	2	-.206	15	.695	5	0	5	0	10	0	15
70		min	-89.596	3	-.869	4	-.102	1	0	1	0	4	0	4
71	17	max	102.808	2	-.247	15	.829	5	0	5	0	10	0	15
72		min	-89.647	3	-1.047	4	-.102	1	0	1	0	4	0	4
73	18	max	102.741	2	-.289	15	.962	5	0	5	0	10	0	15
74		min	-89.697	3	-1.224	4	-.102	1	0	1	0	4	0	4
75	19	max	102.673	2	-.331	15	1.096	5	0	5	0	5	0	1
76		min	-89.748	3	-1.402	4	-.102	1	0	1	0	1	0	1
77	M4	1	max	292.011	1	0	.051	10	0	1	0	5	0	1
78		min	-74.963	3	0	1	-13.203	4	0	1	0	2	0	1
79	2	max	292.075	1	0	1	.051	10	0	1	0	10	0	1
80		min	-74.915	3	0	1	-13.259	4	0	1	-.001	4	0	1
81	3	max	292.14	1	0	1	.051	10	0	1	0	10	0	1
82		min	-74.866	3	0	1	-13.315	4	0	1	-.002	4	0	1
83	4	max	292.205	1	0	1	.051	10	0	1	0	10	0	1
84		min	-74.818	3	0	1	-13.371	4	0	1	-.004	4	0	1
85	5	max	292.269	1	0	1	.051	10	0	1	0	10	0	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143		15	max	660.343	1	.129	2	.013	9	0	3	0	4	0	15
144			min	-1061.887	3	-.022	3	-.488	5	0	5	0	3	0	2
145		16	max	660.45	1	.097	2	.013	9	0	3	0	4	0	15
146			min	-1061.807	3	-.046	3	-.585	5	0	5	0	3	0	2
147		17	max	660.556	1	.065	2	.013	9	0	3	0	4	0	15
148			min	-1061.727	3	-.07	3	-.681	5	0	5	0	3	0	2
149		18	max	660.663	1	.032	2	.013	9	0	3	0	4	0	15
150			min	-1061.647	3	-.094	3	-.778	5	0	5	0	3	0	2
151		19	max	660.769	1	0	2	.013	9	0	3	0	14	0	15
152			min	-1061.567	3	-.118	3	-.874	5	0	5	0	3	0	2
153	M7	1	max	310.456	2	1.807	4	.013	3	0	9	0	4	0	2
154			min	-216.523	3	.429	15	-1.373	4	0	3	0	3	0	12
155		2	max	310.388	2	1.629	4	.013	3	0	9	0	4	0	2
156			min	-216.573	3	.387	15	-1.239	4	0	3	0	3	0	12
157		3	max	310.32	2	1.452	4	.013	3	0	9	0	9	0	2
158			min	-216.624	3	.346	15	-1.105	4	0	3	0	3	0	3
159		4	max	310.253	2	1.274	4	.013	3	0	9	0	9	0	2
160			min	-216.675	3	.304	15	-.972	4	0	3	0	3	0	3
161		5	max	310.185	2	1.096	4	.013	3	0	9	0	9	0	15
162			min	-216.726	3	.262	15	-.838	4	0	3	0	5	0	6
163		6	max	310.117	2	.919	4	.013	3	0	9	0	9	0	15
164			min	-216.777	3	.22	15	-.705	4	0	3	0	5	0	6
165		7	max	310.049	2	.741	4	.013	3	0	9	0	9	0	15
166			min	-216.828	3	.178	15	-.571	4	0	3	0	5	0	6
167		8	max	309.981	2	.563	4	.013	3	0	9	0	9	0	15
168			min	-216.879	3	.137	15	-.437	4	0	3	0	5	0	6
169		9	max	309.913	2	.386	4	.013	3	0	9	0	9	0	15
170			min	-216.93	3	.095	15	-.304	4	0	3	0	5	-.001	6
171		10	max	309.845	2	.209	2	.013	3	0	9	0	9	0	15
172			min	-216.981	3	.051	12	-.17	4	0	3	0	5	-.001	6
173		11	max	309.778	2	.071	2	.013	3	0	9	0	9	0	15
174			min	-217.032	3	-.03	3	-.036	4	0	3	0	5	-.001	6
175		12	max	309.71	2	-.03	15	.099	5	0	9	0	9	0	15
176			min	-217.082	3	-.147	6	-.004	9	0	3	0	5	-.001	6
177		13	max	309.642	2	-.072	15	.233	5	0	9	0	9	0	15
178			min	-217.133	3	-.325	6	-.004	9	0	3	0	5	-.001	6
179		14	max	309.574	2	-.114	15	.366	5	0	9	0	9	0	15
180			min	-217.184	3	-.503	6	-.004	9	0	3	0	5	-.001	6
181		15	max	309.506	2	-.156	15	.5	5	0	9	0	9	0	15
182			min	-217.235	3	-.68	6	-.004	9	0	3	0	5	0	6
183		16	max	309.438	2	-.197	15	.634	5	0	9	0	9	0	15
184			min	-217.286	3	-.858	6	-.004	9	0	3	0	5	0	6
185		17	max	309.37	2	-.239	15	.767	5	0	9	0	9	0	15
186			min	-217.337	3	-1.036	6	-.004	9	0	3	0	5	0	6
187		18	max	309.303	2	-.281	15	.901	5	0	9	0	9	0	15
188			min	-217.388	3	-1.213	6	-.004	9	0	3	0	3	0	6
189		19	max	309.235	2	-.323	15	1.034	5	0	9	0	9	0	1
190			min	-217.439	3	-1.391	6	-.004	9	0	3	0	3	0	1
191	M8	1	max	888.493	2	0	1	.061	9	0	1	0	4	0	1
192			min	-268.936	3	0	1	-13.51	4	0	1	0	3	0	1
193		2	max	888.557	2	0	1	.061	9	0	1	0	9	0	1
194			min	-268.887	3	0	1	-13.566	4	0	1	-.001	4	0	1
195		3	max	888.622	2	0	1	.061	9	0	1	0	9	0	1
196			min	-268.839	3	0	1	-13.623	4	0	1	-.002	4	0	1
197		4	max	888.687	2	0	1	.061	9	0	1	0	9	0	1
198			min	-268.79	3	0	1	-13.679	4	0	1	-.004	4	0	1
199		5	max	888.752	2	0	1	.061	9	0	1	0	9	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-268.742	3	0	1	-13.735	4	0	1	-.005	4	0	1
201		6	max	888.816	2	0	1	.061	9	0	1	0	9	0	1
202			min	-268.693	3	0	1	-13.791	4	0	1	-.006	4	0	1
203		7	max	888.881	2	0	1	.061	9	0	1	0	9	0	1
204			min	-268.645	3	0	1	-13.847	4	0	1	-.007	4	0	1
205		8	max	888.946	2	0	1	.061	9	0	1	0	9	0	1
206			min	-268.596	3	0	1	-13.903	4	0	1	-.009	4	0	1
207		9	max	889.01	2	0	1	.061	9	0	1	0	9	0	1
208			min	-268.548	3	0	1	-13.959	4	0	1	-.01	4	0	1
209		10	max	889.075	2	0	1	.061	9	0	1	0	9	0	1
210			min	-268.499	3	0	1	-14.015	4	0	1	-.011	4	0	1
211		11	max	889.14	2	0	1	.061	9	0	1	0	9	0	1
212			min	-268.451	3	0	1	-14.071	4	0	1	-.012	4	0	1
213		12	max	889.204	2	0	1	.061	9	0	1	0	9	0	1
214			min	-268.402	3	0	1	-14.127	4	0	1	-.014	4	0	1
215		13	max	889.269	2	0	1	.061	9	0	1	0	9	0	1
216			min	-268.353	3	0	1	-14.183	4	0	1	-.015	4	0	1
217		14	max	889.334	2	0	1	.061	9	0	1	0	9	0	1
218			min	-268.305	3	0	1	-14.239	4	0	1	-.016	4	0	1
219		15	max	889.399	2	0	1	.061	9	0	1	0	9	0	1
220			min	-268.256	3	0	1	-14.295	4	0	1	-.017	4	0	1
221		16	max	889.463	2	0	1	.061	9	0	1	0	9	0	1
222			min	-268.208	3	0	1	-14.352	4	0	1	-.019	4	0	1
223		17	max	889.528	2	0	1	.061	9	0	1	0	9	0	1
224			min	-268.159	3	0	1	-14.408	4	0	1	-.02	4	0	1
225		18	max	889.593	2	0	1	.061	9	0	1	0	9	0	1
226			min	-268.111	3	0	1	-14.464	4	0	1	-.021	4	0	1
227		19	max	889.657	2	0	1	.061	9	0	1	0	9	0	1
228			min	-268.062	3	0	1	-14.52	4	0	1	-.023	4	0	1
229	M10	1	max	214.751	1	.675	4	.973	5	0	1	0	14	0	1
230			min	-293.319	3	.17	15	-.07	1	-.001	5	0	3	0	1
231		2	max	214.858	1	.634	4	.876	5	0	1	0	4	0	15
232			min	-293.239	3	.161	15	-.07	1	-.001	5	0	3	0	4
233		3	max	214.964	1	.593	4	.78	5	0	1	0	4	0	15
234			min	-293.159	3	.151	15	-.07	1	-.001	5	0	3	0	4
235		4	max	215.071	1	.551	4	.683	5	0	1	0	4	0	15
236			min	-293.079	3	.141	15	-.07	1	-.001	5	0	3	0	4
237		5	max	215.178	1	.51	4	.587	5	0	1	0	4	0	15
238			min	-292.999	3	.132	15	-.07	1	-.001	5	0	3	0	4
239		6	max	215.284	1	.469	4	.491	5	0	1	0	5	0	15
240			min	-292.919	3	.122	15	-.07	1	-.001	5	0	3	0	4
241		7	max	215.391	1	.428	4	.394	5	0	1	0	5	0	15
242			min	-292.839	3	.112	15	-.07	1	-.001	5	0	3	0	4
243		8	max	215.497	1	.386	4	.298	5	0	1	0	5	0	15
244			min	-292.759	3	.103	15	-.07	1	-.001	5	0	3	0	4
245		9	max	215.604	1	.345	4	.201	5	0	1	0	5	0	15
246			min	-292.679	3	.093	15	-.07	1	-.001	5	0	3	0	4
247		10	max	215.71	1	.304	4	.105	5	0	1	0	5	0	15
248			min	-292.599	3	.083	15	-.07	1	-.001	5	0	3	0	4
249		11	max	215.817	1	.263	4	.008	5	0	1	0	5	0	15
250			min	-292.519	3	.073	15	-.07	1	-.001	5	0	3	0	4
251		12	max	215.923	1	.221	4	.006	10	0	1	0	5	0	15
252			min	-292.44	3	.064	15	-.1	4	-.001	5	0	3	0	4
253		13	max	216.03	1	.18	4	.006	10	0	1	0	5	0	15
254			min	-292.36	3	.054	15	-.197	4	-.001	5	0	3	0	4
255		14	max	216.136	1	.139	4	.006	10	0	1	0	5	0	15
256			min	-292.28	3	.041	12	-.293	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	216.243	1	.098	4	.006	10	0	1	0	5	0	15
258		min	-292.2	3	.025	12	-.39	4	-.001	5	0	3	0	4
259	16	max	216.349	1	.056	4	.006	10	0	1	0	5	0	15
260		min	-292.12	3	.009	12	-.486	4	-.001	5	0	3	0	4
261	17	max	216.456	1	.023	5	.006	10	0	1	0	5	0	15
262		min	-292.04	3	-.015	9	-.583	4	-.001	5	0	3	0	4
263	18	max	216.563	1	.008	5	.006	10	0	1	0	5	0	15
264		min	-291.96	3	-.041	9	-.679	4	-.001	5	0	3	0	4
265	19	max	216.669	1	-.004	15	.006	10	0	1	0	5	0	15
266		min	-291.88	3	-.071	1	-.776	4	-.001	5	0	3	0	4
267	M11	1	max	103.523	2	1.792	.102	1	0	4	0	5	0	6
268		min	-89.588	3	.418	15	-1.263	5	0	10	0	1	0	15
269	2	max	103.456	2	1.615	6	.102	1	0	4	0	5	0	2
270		min	-89.639	3	.377	15	-1.13	5	0	10	0	1	0	15
271	3	max	103.388	2	1.437	6	.102	1	0	4	0	3	0	2
272		min	-89.69	3	.335	15	-.996	5	0	10	0	1	0	3
273	4	max	103.32	2	1.259	6	.102	1	0	4	0	3	0	15
274		min	-89.741	3	.293	15	-.862	5	0	10	0	1	0	4
275	5	max	103.252	2	1.082	6	.102	1	0	4	0	3	0	15
276		min	-89.792	3	.251	15	-.729	5	0	10	0	1	0	4
277	6	max	103.184	2	.904	6	.102	1	0	4	0	3	0	15
278		min	-89.843	3	.209	15	-.595	5	0	10	0	4	0	4
279	7	max	103.116	2	.726	6	.102	1	0	4	0	3	0	15
280		min	-89.894	3	.168	15	-.461	5	0	10	0	4	0	4
281	8	max	103.048	2	.549	6	.102	1	0	4	0	3	0	15
282		min	-89.945	3	.126	15	-.328	5	0	10	0	4	-.001	4
283	9	max	102.981	2	.371	6	.102	1	0	4	0	3	0	15
284		min	-89.995	3	.084	15	-.194	5	0	10	0	4	-.001	4
285	10	max	102.913	2	.194	6	.102	1	0	4	0	3	0	15
286		min	-90.046	3	.042	15	-.061	5	0	10	0	4	-.001	4
287	11	max	102.845	2	.037	2	.102	1	0	4	0	3	0	15
288		min	-90.097	3	-.013	3	-.038	3	0	10	0	4	-.001	4
289	12	max	102.777	2	-.041	15	.231	4	0	4	0	3	0	15
290		min	-90.148	3	-.162	4	-.038	3	0	10	0	4	-.001	4
291	13	max	102.709	2	-.083	15	.365	4	0	4	0	3	0	15
292		min	-90.199	3	-.34	4	-.038	3	0	10	0	4	-.001	4
293	14	max	102.641	2	-.125	15	.498	4	0	4	0	3	0	15
294		min	-90.25	3	-.517	4	-.038	3	0	10	0	5	-.001	4
295	15	max	102.573	2	-.166	15	.632	4	0	4	0	3	0	15
296		min	-90.301	3	-.695	4	-.038	3	0	10	0	5	0	4
297	16	max	102.506	2	-.208	15	.766	4	0	4	0	3	0	15
298		min	-90.352	3	-.873	4	-.038	3	0	10	0	5	0	4
299	17	max	102.438	2	-.25	15	.899	4	0	4	0	3	0	15
300		min	-90.403	3	-1.05	4	-.038	3	0	10	0	10	0	4
301	18	max	102.37	2	-.292	15	1.033	4	0	4	0	4	0	15
302		min	-90.454	3	-1.228	4	-.038	3	0	10	0	10	0	4
303	19	max	102.302	2	-.333	15	1.166	4	0	4	0	4	0	1
304		min	-90.504	3	-1.406	4	-.038	3	0	10	0	10	0	1
305	M12	1	max	292.274	1	0	.393	3	0	1	0	4	0	1
306		min	-74.483	3	0	1	-12.434	5	0	1	0	3	0	1
307	2	max	292.338	1	0	1	.393	3	0	1	0	1	0	1
308		min	-74.435	3	0	1	-12.49	5	0	1	-.001	5	0	1
309	3	max	292.403	1	0	1	.393	3	0	1	0	1	0	1
310		min	-74.386	3	0	1	-12.547	5	0	1	-.002	5	0	1
311	4	max	292.468	1	0	1	.393	3	0	1	0	1	0	1
312		min	-74.338	3	0	1	-12.603	5	0	1	-.003	5	0	1
313	5	max	292.533	1	0	1	.393	3	0	1	0	1	0	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371	15	max	43.651	1	1.158	9	1.169	10	0	5	.001	10	.145	2
372		min	-2.209	10	-21.099	3	-10.026	1	0	1	-.011	4	-.092	3
373	16	max	79.597	2	52.824	2	1.18	10	0	1	.002	10	.149	2
374		min	-30.977	3	-86.482	3	-10.12	1	0	5	-.013	1	-.087	3
375	17	max	79.692	2	52.561	2	1.18	10	0	1	.002	10	.137	2
376		min	-30.905	3	-86.679	3	-10.12	1	0	5	-.015	1	-.068	3
377	18	max	-2.38	10	318.41	2	1.23	10	0	3	.002	10	.069	2
378		min	-53.824	1	-157.386	3	-20.89	4	0	2	-.019	4	-.034	3
379	19	max	-2.301	10	318.147	2	1.23	10	0	3	.002	10	0	2
380		min	-53.729	1	-157.583	3	-20.648	4	0	2	-.024	4	0	3
381	M5	1	max	139.838	1	1054.94	3	0	1	0	.027	4	0	3
382		min	-6.211	3	-690.256	2	-68.401	3	0	3	0	11	0	2
383	2	max	139.933	1	1054.743	3	0	1	0	9	.023	4	.149	2
384		min	-6.14	3	-690.519	2	-68.401	3	0	3	-.004	3	-.228	3
385	3	max	103.854	1	5.388	9	7.216	3	0	3	.019	4	.296	2
386		min	-1.091	5	-67.618	3	-15.182	4	0	4	-.018	3	-.452	3
387	4	max	103.95	1	5.169	9	7.216	3	0	3	.016	4	.309	2
388		min	-1.046	5	-67.815	3	-14.94	4	0	4	-.017	3	-.437	3
389	5	max	104.045	1	4.95	9	7.216	3	0	3	.013	4	.322	2
390		min	-1.002	5	-68.012	3	-14.698	4	0	4	-.015	3	-.422	3
391	6	max	104.141	1	4.732	9	7.216	3	0	3	.01	4	.334	2
392		min	-.957	5	-68.209	3	-14.456	4	0	4	-.014	3	-.408	3
393	7	max	104.237	1	4.513	9	7.216	3	0	3	.007	4	.347	2
394		min	-.913	5	-68.405	3	-14.214	4	0	4	-.012	3	-.393	3
395	8	max	104.332	1	4.294	9	7.216	3	0	3	.004	4	.36	2
396		min	-.868	5	-68.602	3	-13.972	4	0	4	-.01	3	-.378	3
397	9	max	104.428	1	4.076	9	7.216	3	0	3	0	4	.373	2
398		min	-.824	5	-68.799	3	-13.73	4	0	4	-.009	3	-.363	3
399	10	max	104.523	1	3.857	9	7.216	3	0	3	0	1	.386	2
400		min	-.779	5	-68.996	3	-13.488	4	0	4	-.007	3	-.348	3
401	11	max	104.619	1	3.638	9	7.216	3	0	3	0	1	.399	2
402		min	-.735	5	-69.193	3	-13.246	4	0	4	-.006	3	-.333	3
403	12	max	104.714	1	3.42	9	7.216	3	0	3	0	1	.412	2
404		min	-.69	5	-69.389	3	-13.004	4	0	4	-.008	4	-.318	3
405	13	max	104.81	1	3.201	9	7.216	3	0	3	0	1	.425	2
406		min	-.645	5	-69.586	3	-12.762	4	0	4	-.011	4	-.303	3
407	14	max	104.905	1	2.982	9	7.216	3	0	3	0	1	.438	2
408		min	-.601	5	-69.783	3	-12.52	4	0	4	-.014	4	-.288	3
409	15	max	105.001	1	2.764	9	7.216	3	0	3	0	3	.451	2
410		min	-.556	5	-69.98	3	-12.278	4	0	4	-.016	4	-.273	3
411	16	max	243.045	2	163.28	2	7.19	3	0	3	.002	3	.463	2
412		min	-92.328	3	-226.79	3	-11.007	4	0	4	-.019	4	-.256	3
413	17	max	243.14	2	163.018	2	7.19	3	0	3	.003	3	.427	2
414		min	-92.257	3	-226.987	3	-10.765	4	0	4	-.022	4	-.207	3
415	18	max	-.441	3	994.242	2	6.656	3	0	4	.005	3	.215	2
416		min	-140.022	1	-480.652	3	-23.797	5	0	9	-.027	4	-.104	3
417	19	max	-.369	3	993.98	2	6.656	3	0	4	.006	3	0	3
418		min	-139.927	1	-480.849	3	-23.555	5	0	9	-.032	4	0	2
419	M9	1	max	53.773	1	334.628	3	100.234	4	0	.002	10	0	2
420		min	-.202	15	-222.147	2	-1.174	10	0	2	-.02	1	0	3
421	2	max	53.868	1	334.432	3	100.476	4	0	3	.021	5	.049	2
422		min	-.174	15	-222.409	2	-1.174	10	0	2	-.018	1	-.073	3
423	3	max	42.852	1	3.773	9	10.026	1	0	1	.041	5	.096	2
424		min	-2.875	10	-18.633	3	-19.631	5	0	10	-.015	1	-.144	3
425	4	max	42.948	1	3.555	9	10.026	1	0	1	.037	5	.1	2
426		min	-2.795	10	-18.83	3	-19.389	5	0	10	-.013	1	-.14	3
427	5	max	43.043	1	3.336	9	10.026	1	0	1	.033	5	.104	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428		min	-2.715	10	-19.027	3	-19.147	5	0	10	-.011	1	-.136	3
429	6	max	43.139	1	3.117	9	10.026	1	0	1	.029	5	.107	2
430		min	-2.636	10	-19.223	3	-18.905	5	0	10	-.009	1	-.132	3
431	7	max	43.234	1	2.899	9	10.026	1	0	1	.025	5	.111	2
432		min	-2.556	10	-19.42	3	-18.663	5	0	10	-.007	1	-.127	3
433	8	max	43.33	1	2.68	9	10.026	1	0	1	.021	5	.115	2
434		min	-2.477	10	-19.617	3	-18.421	5	0	10	-.004	1	-.123	3
435	9	max	43.425	1	2.461	9	10.026	1	0	1	.017	5	.119	2
436		min	-2.397	10	-19.814	3	-18.179	5	0	10	-.002	1	-.119	3
437	10	max	43.521	1	2.243	9	10.026	1	0	1	.013	4	.124	2
438		min	-2.318	10	-20.011	3	-17.937	5	0	10	0	1	-.114	3
439	11	max	43.616	1	2.024	9	10.026	1	0	1	.009	4	.128	2
440		min	-2.238	10	-20.207	3	-17.695	5	0	10	0	10	-.11	3
441	12	max	43.712	1	1.805	9	10.026	1	0	1	.008	3	.132	2
442		min	-2.158	10	-20.404	3	-17.453	5	0	10	0	10	-.106	3
443	13	max	43.807	1	1.587	9	10.026	1	0	1	.008	3	.136	2
444		min	-2.079	10	-20.601	3	-17.211	5	0	10	0	10	-.101	3
445	14	max	43.903	1	1.368	9	10.026	1	0	1	.009	1	.141	2
446		min	-1.999	10	-20.798	3	-16.969	5	0	10	-.002	5	-.097	3
447	15	max	43.998	1	1.149	9	10.026	1	0	1	.011	1	.145	2
448		min	-1.92	10	-20.995	3	-16.727	5	0	10	-.006	5	-.092	3
449	16	max	79.687	2	52.577	2	10.12	1	0	10	.013	1	.149	2
450		min	-32.063	3	-86.925	3	-15.344	5	0	4	-.009	5	-.087	3
451	17	max	79.782	2	52.315	2	10.12	1	0	10	.015	1	.137	2
452		min	-31.991	3	-87.121	3	-15.102	5	0	4	-.012	5	-.068	3
453	18	max	9.34	5	318.41	2	10.488	1	0	2	.018	1	.069	2
454		min	-53.824	1	-157.378	3	-27.292	5	0	3	-.018	5	-.034	3
455	19	max	9.385	5	318.147	2	10.488	1	0	2	.02	1	0	2
456		min	-53.728	1	-157.574	3	-27.05	5	0	3	-.024	5	0	3
457	M13	1	max	100.234	4	222.094	2	.202	15	0	.02	1	0	2
458		min	-1.174	10	-334.675	3	-53.771	1	0	3	-.002	10	0	3
459	2	max	96.414	4	159.102	2	.895	5	0	2	.014	3	.112	3
460		min	-1.174	10	-238.882	3	-40.027	1	0	3	-.004	2	-.074	2
461	3	max	92.595	4	96.11	2	1.598	5	0	2	.011	3	.186	3
462		min	-1.174	10	-143.089	3	-26.284	1	0	3	-.011	1	-.124	2
463	4	max	88.775	4	33.118	2	2.302	5	0	2	.008	3	.223	3
464		min	-1.174	10	-47.296	3	-12.54	1	0	3	-.019	1	-.149	2
465	5	max	84.956	4	48.497	3	4.088	2	0	2	.005	3	.223	3
466		min	-1.174	10	-29.976	1	-5.981	3	0	3	-.021	1	-.149	2
467	6	max	81.136	4	144.29	3	14.947	1	0	2	.004	5	.185	3
468		min	-1.174	10	-92.865	2	-5.32	3	0	3	-.018	1	-.126	2
469	7	max	77.317	4	240.083	3	28.691	1	0	2	.005	5	.11	3
470		min	-1.174	10	-155.857	2	-4.658	3	0	3	-.009	1	-.077	2
471	8	max	73.497	4	335.877	3	42.434	1	0	2	.008	4	0	5
472		min	-1.174	10	-218.849	2	-3.996	3	0	3	0	3	-.004	2
473	9	max	72.09	3	431.67	3	56.178	1	0	2	.024	1	.093	1
474		min	-1.174	10	-281.841	2	-3.335	3	0	3	-.002	3	-.151	3
475	10	max	72.09	3	10.708	5	69.922	1	0	2	.048	1	.215	2
476		min	-1.174	10	-527.463	3	2.09	12	0	3	-.015	5	-.337	3
477	11	max	44.458	4	281.841	2	7.518	5	0	3	.024	1	.093	1
478		min	-1.174	10	-431.67	3	-56.178	1	0	2	-.013	5	-.151	3
479	12	max	40.638	4	218.849	2	8.221	5	0	3	.006	2	0	5
480		min	-1.174	10	-335.876	3	-42.434	1	0	2	-.01	3	-.004	2
481	13	max	36.819	4	155.857	2	8.925	5	0	3	.001	10	.11	3
482		min	-1.174	10	-240.083	3	-28.69	1	0	2	-.009	1	-.077	2
483	14	max	32.999	4	92.865	2	9.628	5	0	3	0	10	.185	3
484		min	-1.174	10	-144.29	3	-14.947	1	0	2	-.018	1	-.126	2



RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 20° 130mph 30psf 3.5ft 7-05.rdb Page 30



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542		min	-96.702	3	0	1	0	1	0	3	0	3	0	3
543	6	max	0	1	.323	3	.169	3	0	1	0	1	0	1
544		min	-96.762	3	0	1	0	1	0	3	0	3	0	3
545	7	max	0	1	.242	3	.169	3	0	1	0	3	0	1
546		min	-96.822	3	0	1	0	1	0	3	0	1	0	3
547	8	max	0	1	.162	3	.169	3	0	1	0	3	0	1
548		min	-96.881	3	0	1	0	1	0	3	0	1	0	3
549	9	max	0	1	.081	3	.169	3	0	1	0	3	0	1
550		min	-96.941	3	0	1	0	1	0	3	0	1	0	3
551	10	max	0	1	0	1	.169	3	0	1	0	3	0	1
552		min	-97.001	3	0	1	0	1	0	3	0	1	0	3
553	11	max	0	1	0	1	.169	3	0	1	0	3	0	1
554		min	-97.06	3	-.081	3	0	1	0	3	0	1	0	3
555	12	max	0	1	0	1	.169	3	0	1	0	3	0	1
556		min	-97.12	3	-.162	3	0	1	0	3	0	1	0	3
557	13	max	0	1	0	1	.169	3	0	1	0	3	0	1
558		min	-97.18	3	-.242	3	0	1	0	3	0	1	0	3
559	14	max	0	1	0	1	.169	3	0	1	0	3	0	1
560		min	-97.239	3	-.323	3	0	1	0	3	0	1	0	3
561	15	max	0	1	0	1	.169	3	0	1	0	3	0	1
562		min	-97.299	3	-.404	3	0	1	0	3	0	1	0	3
563	16	max	0	1	0	1	.169	3	0	1	0	3	0	1
564		min	-97.359	3	-.485	3	0	1	0	3	0	1	0	3
565	17	max	0	1	0	1	.169	3	0	1	0	3	0	1
566		min	-97.418	3	-.565	3	0	1	0	3	0	1	0	3
567	18	max	0	1	0	1	.169	3	0	1	0	3	0	1
568		min	-97.478	3	-.646	3	0	1	0	3	0	1	0	3
569	19	max	0	1	0	1	.169	3	0	1	0	3	0	1
570		min	-97.538	3	-.727	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	1	1.849	.269	4	0	3	0	3	0	1
572		min	-153.23	4	0	1	-.068	3	0	4	0	4	0	1
573	2	max	0	1	1.644	4	.243	4	0	3	0	3	0	1
574		min	-153.219	4	0	1	-.068	3	0	4	0	4	0	4
575	3	max	0	1	1.438	4	.218	4	0	3	0	3	0	1
576		min	-153.208	4	0	1	-.068	3	0	4	0	4	0	4
577	4	max	0	1	1.233	4	.192	4	0	3	0	3	0	1
578		min	-153.197	4	0	1	-.068	3	0	4	0	4	-.001	4
579	5	max	0	1	1.027	4	.166	4	0	3	0	3	0	1
580		min	-153.186	4	0	1	-.068	3	0	4	0	9	-.001	4
581	6	max	0	1	.822	4	.14	4	0	3	0	3	0	1
582		min	-153.175	4	0	1	-.068	3	0	4	0	9	-.002	4
583	7	max	0	1	.616	4	.114	4	0	3	0	3	0	1
584		min	-153.164	4	0	1	-.068	3	0	4	0	9	-.002	4
585	8	max	0	1	.411	4	.088	4	0	3	0	3	0	1
586		min	-153.153	4	0	1	-.068	3	0	4	0	9	-.002	4
587	9	max	0	1	.205	4	.062	4	0	3	0	5	0	1
588		min	-153.141	4	0	1	-.068	3	0	4	0	9	-.002	4
589	10	max	0	1	0	1	.036	4	0	3	0	5	0	1
590		min	-153.13	4	0	1	-.068	3	0	4	0	9	-.002	4
591	11	max	0	1	0	1	.017	9	0	3	0	5	0	1
592		min	-153.119	4	-.205	4	-.068	3	0	4	0	9	-.002	4
593	12	max	0	1	0	1	.017	9	0	3	0	5	0	1
594		min	-153.108	4	-.411	4	-.068	3	0	4	0	9	-.002	4
595	13	max	0	1	0	1	.017	9	0	3	0	5	0	1
596		min	-153.097	4	-.616	4	-.068	3	0	4	0	3	-.002	4
597	14	max	0	1	0	1	.017	9	0	3	0	5	0	1
598		min	-153.086	4	-.822	4	-.071	5	0	4	0	3	-.002	4





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
599	15	max	.055	9	0	1	.017	9	0	3	0	5	0	1
600		min	-153.075	4	-1.027	4	-.097	5	0	4	0	3	-.001	4
601	16	max	.121	9	0	1	.017	9	0	3	0	4	0	1
602		min	-153.108	5	-1.233	4	-.123	5	0	4	0	3	-.001	4
603	17	max	.187	9	0	1	.017	9	0	3	0	9	0	1
604		min	-153.162	5	-1.438	4	-.149	5	0	4	0	3	0	4
605	18	max	.254	9	0	1	.017	9	0	3	0	9	0	1
606		min	-153.216	5	-1.644	4	-.174	5	0	4	0	3	0	4
607	19	max	.32	9	0	1	.017	9	0	3	0	9	0	1
608		min	-153.27	5	-1.849	4	-.2	5	0	4	0	3	0	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.002	1	.007	2	.001	9	7.514e-4	5	NC	3	NC	1
2			min	-.003	3	-.006	3	-.008	5	-1.686e-4	3	5013.653	2	NC	1
3		2	max	.002	1	.006	2	.001	9	7.706e-4	5	NC	3	NC	1
4			min	-.003	3	-.006	3	-.008	5	-1.599e-4	3	5456.653	2	NC	1
5		3	max	.002	1	.006	2	.001	9	7.898e-4	5	NC	1	NC	1
6			min	-.003	3	-.006	3	-.007	5	-1.511e-4	3	5981.018	2	NC	1
7		4	max	.001	1	.005	2	0	9	8.09e-4	5	NC	1	NC	1
8			min	-.002	3	-.005	3	-.007	5	-1.423e-4	3	6606.184	2	NC	1
9		5	max	.001	1	.005	2	0	9	8.282e-4	5	NC	1	NC	1
10			min	-.002	3	-.005	3	-.007	5	-1.335e-4	3	7358.004	2	NC	1
11		6	max	.001	1	.004	2	0	9	8.474e-4	5	NC	1	NC	1
12			min	-.002	3	-.005	3	-.006	5	-1.247e-4	3	8271.444	2	NC	1
13		7	max	.001	1	.004	2	0	9	8.666e-4	5	NC	1	NC	1
14			min	-.002	3	-.005	3	-.006	5	-1.159e-4	3	9394.723	2	NC	1
15		8	max	.001	1	.003	2	0	9	8.858e-4	5	NC	1	NC	1
16			min	-.002	3	-.004	3	-.006	5	-1.071e-4	3	NC	1	NC	1
17		9	max	0	1	.003	2	0	9	9.05e-4	5	NC	1	NC	1
18			min	-.002	3	-.004	3	-.005	5	-9.844e-5	1	NC	1	NC	1
19		10	max	0	1	.002	2	0	9	9.242e-4	5	NC	1	NC	1
20		min	-.001	3	-.004	3	-.005	5	-9.135e-5	1	NC	1	NC	1	
21	11	max	0	1	.002	2	0	9	9.434e-4	5	NC	1	NC	1	
22		min	-.001	3	-.003	3	-.004	5	-8.426e-5	1	NC	1	NC	1	
23	12	max	0	1	.002	2	0	9	9.627e-4	5	NC	1	NC	1	
24		min	-.001	3	-.003	3	-.004	5	-7.717e-5	1	NC	1	NC	1	
25	13	max	0	1	.001	2	0	9	9.819e-4	5	NC	1	NC	1	
26		min	0	3	-.003	3	-.003	5	-7.008e-5	1	NC	1	NC	1	
27	14	max	0	1	0	2	0	9	1.001e-3	5	NC	1	NC	1	
28		min	0	3	-.002	3	-.003	5	-6.299e-5	1	NC	1	NC	1	
29	15	max	0	1	0	2	0	9	1.02e-3	5	NC	1	NC	1	
30		min	0	3	-.002	3	-.002	5	-5.59e-5	1	NC	1	NC	1	
31	16	max	0	1	0	2	0	1	1.039e-3	5	NC	1	NC	1	
32		min	0	3	-.001	3	-.002	5	-4.881e-5	1	NC	1	NC	1	
33	17	max	0	1	0	2	0	1	1.059e-3	5	NC	1	NC	1	
34		min	0	3	0	3	-.001	5	-4.172e-5	1	NC	1	NC	1	
35	18	max	0	1	0	2	0	1	1.078e-3	5	NC	1	NC	1	
36		min	0	3	0	3	0	5	-3.463e-5	1	NC	1	NC	1	
37	19	max	0	1	0	1	0	1	1.097e-3	5	NC	1	NC	1	
38		min	0	1	0	1	0	1	-2.754e-5	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.273e-5	1	NC	1	NC	1
40			min	0	1	0	1	0	1	-5.042e-4	5	NC	1	NC	1
41		2	max	0	3	0	2	.003	5	1.862e-5	1	NC	1	NC	1
42			min	0	2	0	3	0	9	-5.066e-4	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.005	5	2.451e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	9	-5.091e-4	5	NC	1	NC	1
45		4	max	0	3	0	2	.008	4	3.039e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	9	-5.115e-4	5	NC	1	NC	1
47		5	max	0	3	0	2	.011	4	3.628e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	9	-5.14e-4	5	NC	1	NC	1
49		6	max	0	3	0	2	.013	4	4.217e-5	1	NC	1	NC	1
50			min	0	2	-.004	3	0	9	-5.164e-4	5	NC	1	NC	1
51		7	max	0	3	0	2	.016	4	4.805e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-5.188e-4	5	NC	1	NC	1
53		8	max	0	3	0	2	.018	4	5.394e-5	1	NC	1	NC	1
54			min	0	2	-.005	3	0	10	-5.213e-4	5	NC	1	NC	1
55		9	max	0	3	0	2	.021	4	5.983e-5	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-5.237e-4	5	NC	1	NC	1
57		10	max	0	3	.001	2	.023	4	6.571e-5	1	NC	1	NC	1
58			min	0	2	-.006	3	0	10	-5.262e-4	5	NC	1	NC	1
59		11	max	0	3	.002	2	.025	4	7.16e-5	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-5.286e-4	5	NC	1	NC	1
61		12	max	0	3	.002	2	.028	4	7.749e-5	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-5.31e-4	5	NC	1	NC	1
63		13	max	0	3	.003	2	.03	4	8.337e-5	1	NC	1	NC	1
64			min	0	2	-.007	3	0	10	-5.335e-4	5	NC	1	NC	1
65		14	max	0	3	.004	2	.032	4	8.926e-5	1	NC	1	NC	1
66			min	0	2	-.007	3	0	10	-5.359e-4	5	NC	1	NC	1
67		15	max	0	3	.004	2	.033	4	9.515e-5	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-5.384e-4	5	NC	1	NC	1
69		16	max	0	3	.005	2	.035	4	1.01e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-5.408e-4	5	8921.101	2	NC	1
71		17	max	0	3	.006	2	.037	4	1.069e-4	1	NC	1	NC	1
72			min	-.001	2	-.007	3	0	10	-5.432e-4	5	7596.86	2	NC	1
73		18	max	0	3	.007	2	.039	4	1.128e-4	1	NC	3	NC	1
74			min	-.001	2	-.007	3	0	10	-5.457e-4	5	6577.077	2	NC	1
75		19	max	.001	3	.008	2	.041	4	1.187e-4	1	NC	3	NC	1
76			min	-.001	2	-.007	3	0	10	-5.481e-4	5	5783.085	2	NC	1
77	M4	1	max	.001	1	.007	2	0	10	2.025e-3	5	NC	1	NC	1
78			min	0	3	-.006	3	-.043	4	-1.207e-4	1	NC	1	450.895	4
79		2	max	.001	1	.007	2	0	10	2.025e-3	5	NC	1	NC	1
80			min	0	3	-.006	3	-.039	4	-1.207e-4	1	NC	1	491.462	4
81		3	max	.001	1	.007	2	0	10	2.025e-3	5	NC	1	NC	1
82			min	0	3	-.006	3	-.036	4	-1.207e-4	1	NC	1	539.734	4
83		4	max	.001	1	.006	2	0	10	2.025e-3	5	NC	1	NC	1
84			min	0	3	-.005	3	-.032	4	-1.207e-4	1	NC	1	597.741	4
85		5	max	.001	1	.006	2	0	10	2.025e-3	5	NC	1	NC	1
86			min	0	3	-.005	3	-.029	4	-1.207e-4	1	NC	1	668.248	4
87		6	max	.001	1	.005	2	0	10	2.025e-3	5	NC	1	NC	1
88			min	0	3	-.005	3	-.026	4	-1.207e-4	1	NC	1	755.097	4
89		7	max	0	1	.005	2	0	10	2.025e-3	5	NC	1	NC	1
90			min	0	3	-.004	3	-.022	4	-1.207e-4	1	NC	1	863.764	4
91		8	max	0	1	.005	2	0	10	2.025e-3	5	NC	1	NC	1
92			min	0	3	-.004	3	-.019	4	-1.207e-4	1	NC	1	1002.262	4
93		9	max	0	1	.004	2	0	10	2.025e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.016	4	-1.207e-4	1	NC	1	1182.711	4
95		10	max	0	1	.004	2	0	10	2.025e-3	5	NC	1	NC	1
96			min	0	3	-.003	3	-.014	4	-1.207e-4	1	NC	1	1424.166	4
97		11	max	0	1	.003	2	0	10	2.025e-3	5	NC	1	NC	1
98			min	0	3	-.003	3	-.011	4	-1.207e-4	1	NC	1	1758.001	4
99		12	max	0	1	.003	2	0	10	2.025e-3	5	NC	1	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.002	2	.006	4	4.545e-6	9	NC	1	NC	1
158			min	0	2	-.003	3	0	9	-5.092e-4	4	NC	1	NC	1
159		4	max	0	3	.003	2	.008	4	4.053e-6	9	NC	1	NC	1
160			min	0	2	-.005	3	0	9	-5.016e-4	4	NC	1	NC	1
161		5	max	0	3	.004	2	.011	4	3.561e-6	9	NC	1	NC	1
162			min	0	2	-.006	3	0	9	-4.94e-4	4	NC	1	NC	1
163		6	max	0	3	.005	2	.014	4	1.243e-5	3	NC	1	NC	1
164			min	0	2	-.008	3	0	9	-4.865e-4	4	8802.335	2	NC	1
165		7	max	0	3	.006	2	.017	4	3.356e-5	3	NC	1	NC	1
166			min	-.001	2	-.009	3	0	9	-4.789e-4	4	7295.95	2	NC	1
167		8	max	0	3	.007	2	.019	4	5.47e-5	3	NC	3	NC	1
168			min	-.001	2	-.01	3	0	9	-4.713e-4	4	6185.215	2	NC	1
169		9	max	.001	3	.009	2	.022	4	7.583e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	9	-4.638e-4	4	5327.432	2	NC	1
171		10	max	.001	3	.01	2	.024	4	9.696e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	0	9	-4.562e-4	4	4643.522	2	NC	1
173		11	max	.001	3	.011	2	.026	4	1.181e-4	3	NC	3	NC	1
174			min	-.002	2	-.014	3	0	9	-4.486e-4	4	4085.888	2	NC	1
175		12	max	.002	3	.013	2	.029	4	1.392e-4	3	NC	3	NC	1
176			min	-.002	2	-.015	3	0	9	-4.41e-4	4	3623.889	2	NC	1
177		13	max	.002	3	.014	2	.031	4	1.604e-4	3	NC	3	NC	1
178			min	-.002	2	-.016	3	0	9	-4.335e-4	4	3236.666	2	NC	1
179		14	max	.002	3	.016	2	.033	4	1.815e-4	3	NC	3	NC	1
180			min	-.003	2	-.017	3	0	9	-4.259e-4	4	2909.342	2	NC	1
181		15	max	.002	3	.018	2	.035	4	2.026e-4	3	NC	3	NC	1
182			min	-.003	2	-.018	3	0	9	-4.183e-4	4	2630.899	2	NC	1
183		16	max	.002	3	.019	2	.037	4	2.238e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	0	9	-4.108e-4	4	2392.92	2	NC	1
185		17	max	.002	3	.021	2	.038	4	2.449e-4	3	NC	3	NC	1
186			min	-.003	2	-.019	3	0	9	-4.032e-4	4	2188.834	2	NC	1
187		18	max	.002	3	.023	2	.04	4	2.66e-4	3	NC	3	NC	1
188			min	-.003	2	-.02	3	0	9	-3.956e-4	4	2013.418	2	NC	1
189		19	max	.002	3	.025	2	.042	4	2.871e-4	3	NC	3	NC	1
190			min	-.004	2	-.021	3	0	9	-3.881e-4	4	1862.472	2	NC	1
191	M8	1	max	.004	2	.023	2	0	9	1.881e-3	4	NC	1	NC	1
192			min	-.001	3	-.018	3	-.044	4	-2.175e-4	3	NC	1	440.94	4
193		2	max	.004	2	.022	2	0	9	1.881e-3	4	NC	1	NC	1
194			min	-.001	3	-.017	3	-.04	4	-2.175e-4	3	NC	1	480.613	4
195		3	max	.004	2	.021	2	0	9	1.881e-3	4	NC	1	NC	1
196			min	-.001	3	-.016	3	-.037	4	-2.175e-4	3	NC	1	527.823	4
197		4	max	.004	2	.019	2	0	9	1.881e-3	4	NC	1	NC	1
198			min	-.001	3	-.015	3	-.033	4	-2.175e-4	3	NC	1	584.555	4
199		5	max	.003	2	.018	2	0	9	1.881e-3	4	NC	1	NC	1
200			min	0	3	-.014	3	-.03	4	-2.175e-4	3	NC	1	653.511	4
201		6	max	.003	2	.017	2	0	9	1.881e-3	4	NC	1	NC	1
202			min	0	3	-.013	3	-.026	4	-2.175e-4	3	NC	1	738.451	4
203		7	max	.003	2	.015	2	0	9	1.881e-3	4	NC	1	NC	1
204			min	0	3	-.012	3	-.023	4	-2.175e-4	3	NC	1	844.729	4
205		8	max	.003	2	.014	2	0	9	1.881e-3	4	NC	1	NC	1
206			min	0	3	-.011	3	-.02	4	-2.175e-4	3	NC	1	980.184	4
207		9	max	.002	2	.013	2	0	9	1.881e-3	4	NC	1	NC	1
208			min	0	3	-.01	3	-.017	4	-2.175e-4	3	NC	1	1156.671	4
209		10	max	.002	2	.012	2	0	9	1.881e-3	4	NC	1	NC	1
210			min	0	3	-.009	3	-.014	4	-2.175e-4	3	NC	1	1392.825	4
211		11	max	.002	2	.01	2	0	9	1.881e-3	4	NC	1	NC	1
212			min	0	3	-.008	3	-.011	4	-2.175e-4	3	NC	1	1719.332	4
213		12	max	.002	2	.009	2	0	9	1.881e-3	4	NC	1	NC	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.004	4	5.875e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-4.756e-4	4	NC	1	NC	1
273		4	max	0	3	0	2	.007	4	3.783e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-5.103e-4	4	NC	1	NC	1
275		5	max	0	3	0	2	.009	4	1.691e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.002	3	-5.449e-4	4	NC	1	NC	1
277		6	max	0	3	0	2	.011	4	4.963e-6	10	NC	1	NC	1
278			min	0	2	-.004	3	-.002	3	-5.796e-4	4	NC	1	NC	1
279		7	max	0	3	0	2	.013	4	5.69e-6	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-6.142e-4	4	NC	1	NC	1
281		8	max	0	3	0	2	.015	4	6.417e-6	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-6.489e-4	4	NC	1	NC	1
283		9	max	0	3	0	2	.018	5	7.143e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.003	3	-6.835e-4	4	NC	1	NC	1
285		10	max	0	3	.001	2	.02	5	7.87e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.003	3	-7.182e-4	4	NC	1	NC	1
287		11	max	0	3	.002	2	.022	5	8.596e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.003	3	-7.529e-4	4	NC	1	NC	1
289		12	max	0	3	.002	2	.024	5	9.323e-6	10	NC	1	NC	1
290			min	0	2	-.007	3	-.003	3	-7.875e-4	4	NC	1	NC	1
291		13	max	0	3	.003	2	.026	5	1.005e-5	10	NC	1	NC	1
292			min	0	2	-.007	3	-.003	3	-8.222e-4	4	NC	1	NC	1
293		14	max	0	3	.004	2	.028	5	1.078e-5	10	NC	1	NC	1
294			min	0	2	-.007	3	-.003	3	-8.568e-4	4	NC	1	NC	1
295		15	max	0	3	.004	2	.03	5	1.15e-5	10	NC	1	NC	1
296			min	0	2	-.007	3	-.003	3	-8.915e-4	4	NC	1	NC	1
297		16	max	0	3	.005	2	.031	5	1.223e-5	10	NC	1	NC	1
298			min	0	2	-.007	3	-.003	3	-9.261e-4	4	8931.615	2	NC	1
299		17	max	0	3	.006	2	.033	5	1.296e-5	10	NC	1	NC	1
300			min	-.001	2	-.007	3	-.002	3	-9.608e-4	4	7604.874	2	NC	1
301		18	max	0	3	.007	2	.035	5	1.368e-5	10	NC	3	NC	1
302			min	-.001	2	-.007	3	-.002	3	-9.954e-4	4	6583.373	2	NC	1
303		19	max	.001	3	.008	2	.037	5	1.441e-5	10	NC	3	NC	1
304			min	-.001	2	-.007	3	-.002	3	-1.03e-3	4	5788.175	2	NC	1
305	M12	1	max	.001	1	.007	2	.001	3	2.423e-3	4	NC	1	NC	1
306			min	0	3	-.006	3	-.04	5	-1.483e-5	10	NC	1	478.04	5
307		2	max	.001	1	.007	2	.001	3	2.423e-3	4	NC	1	NC	1
308			min	0	3	-.006	3	-.037	5	-1.483e-5	10	NC	1	521.037	5
309		3	max	.001	1	.007	2	.001	3	2.423e-3	4	NC	1	NC	1
310			min	0	3	-.006	3	-.034	5	-1.483e-5	10	NC	1	572.2	5
311		4	max	.001	1	.006	2	0	3	2.423e-3	4	NC	1	NC	1
312			min	0	3	-.005	3	-.03	5	-1.483e-5	10	NC	1	633.68	5
313		5	max	.001	1	.006	2	0	3	2.423e-3	4	NC	1	NC	1
314			min	0	3	-.005	3	-.027	5	-1.483e-5	10	NC	1	708.406	5
315		6	max	.001	1	.005	2	0	3	2.423e-3	4	NC	1	NC	1
316			min	0	3	-.005	3	-.024	5	-1.483e-5	10	NC	1	800.451	5
317		7	max	0	1	.005	2	0	3	2.423e-3	4	NC	1	NC	1
318			min	0	3	-.004	3	-.021	5	-1.483e-5	10	NC	1	915.616	5
319		8	max	0	1	.005	2	0	3	2.423e-3	4	NC	1	NC	1
320			min	0	3	-.004	3	-.018	5	-1.483e-5	10	NC	1	1062.393	5
321		9	max	0	1	.004	2	0	3	2.423e-3	4	NC	1	NC	1
322			min	0	3	-.004	3	-.015	5	-1.483e-5	10	NC	1	1253.626	5
323		10	max	0	1	.004	2	0	3	2.423e-3	4	NC	1	NC	1
324			min	0	3	-.003	3	-.013	5	-1.483e-5	10	NC	1	1509.505	5
325		11	max	0	1	.003	2	0	3	2.423e-3	4	NC	1	NC	1
326			min	0	3	-.003	3	-.01	5	-1.483e-5	10	NC	1	1863.277	5
327		12	max	0	1	.003	2	0	3	2.423e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.008	5	-1.483e-5	10	NC	1	2372.928	5
329		13	max	0	1	.002	2	0	3	2.423e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.006	5	-1.483e-5	10	NC	1	3147.221	5
331		14	max	0	1	.002	2	0	3	2.423e-3	4	NC	1	NC	1
332			min	0	3	-.002	3	-.004	5	-1.483e-5	10	NC	1	4410.473	5
333		15	max	0	1	.002	2	0	3	2.423e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.003	5	-1.483e-5	10	NC	1	6689.635	5
335		16	max	0	1	.001	2	0	3	2.423e-3	4	NC	1	NC	1
336			min	0	3	-.001	3	-.002	5	-1.483e-5	10	NC	1	NC	1
337		17	max	0	1	0	2	0	3	2.423e-3	4	NC	1	NC	1
338			min	0	3	0	3	0	5	-1.483e-5	10	NC	1	NC	1
339		18	max	0	1	0	2	0	3	2.423e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	-1.483e-5	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.423e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	-1.483e-5	10	NC	1	NC	1
343	M1	1	max	.006	3	.022	3	.005	5	4.788e-3	2	NC	1	NC	1
344			min	-.007	2	-.018	2	0	9	-6.83e-3	3	NC	1	NC	1
345		2	max	.006	3	.012	3	.006	5	2.369e-3	2	NC	4	NC	1
346			min	-.007	2	-.01	2	0	9	-3.347e-3	3	5050.397	3	NC	1
347		3	max	.006	3	.003	3	.008	5	1.821e-4	5	NC	4	NC	1
348			min	-.007	2	-.002	2	-.001	9	-5.838e-5	9	2620.75	3	NC	1
349		4	max	.006	3	.005	2	.01	5	1.757e-4	5	NC	4	NC	1
350			min	-.007	2	-.004	3	-.002	1	-4.769e-5	9	1874.106	3	8619.607	5
351		5	max	.006	3	.011	2	.012	5	1.693e-4	5	NC	4	NC	1
352			min	-.007	2	-.01	3	-.002	1	-3.7e-5	9	1519.388	3	6110.242	5
353		6	max	.006	3	.016	2	.015	5	1.63e-4	5	NC	4	NC	1
354			min	-.007	2	-.015	3	-.001	1	-2.631e-5	9	1322.641	3	4660.388	5
355		7	max	.006	3	.02	2	.017	5	1.566e-4	5	NC	4	NC	1
356			min	-.007	2	-.019	3	-.001	1	-1.562e-5	9	1194.111	2	3730.289	5
357		8	max	.006	3	.022	2	.02	5	1.503e-4	5	NC	4	NC	1
358			min	-.007	2	-.021	3	0	9	-4.928e-6	9	1104.029	2	3091.162	5
359		9	max	.006	3	.024	2	.022	5	1.471e-4	4	NC	4	NC	1
360			min	-.007	2	-.023	3	0	9	-2.268e-6	10	1051.005	2	2630.108	5
361		10	max	.006	3	.025	2	.025	4	1.442e-4	4	NC	4	NC	1
362			min	-.007	2	-.023	3	0	9	-3.864e-6	10	1026.971	2	2272.345	4
363		11	max	.006	3	.025	2	.028	4	1.412e-4	4	NC	4	NC	1
364			min	-.007	2	-.022	3	0	10	-5.46e-6	10	1029.053	2	1999.519	4
365		12	max	.006	3	.023	2	.031	4	1.382e-4	4	NC	4	NC	1
366			min	-.007	2	-.02	3	0	10	-7.055e-6	10	1058.492	2	1787.456	4
367		13	max	.006	3	.02	2	.033	4	1.353e-4	4	NC	4	NC	1
368			min	-.007	2	-.018	3	0	10	-8.651e-6	10	1121.292	2	1619.886	4
369		14	max	.006	3	.016	2	.036	4	1.323e-4	4	NC	4	NC	1
370			min	-.007	2	-.014	3	0	10	-1.025e-5	10	1231.24	2	1485.865	4
371		15	max	.006	3	.011	2	.038	4	1.294e-4	4	NC	4	NC	1
372			min	-.007	2	-.009	3	0	10	-1.184e-5	10	1418.498	2	1377.821	4
373		16	max	.006	3	.004	2	.041	4	2.653e-4	4	NC	4	NC	1
374			min	-.007	2	-.004	3	0	10	-1.305e-5	10	1756.468	2	1290.383	4
375		17	max	.006	3	.002	3	.043	4	3.708e-3	4	NC	4	NC	1
376			min	-.007	2	-.004	2	0	10	-5.018e-6	10	2476.792	2	1219.766	4
377		18	max	.006	3	.009	3	.045	4	3.322e-3	2	NC	4	NC	1
378			min	-.007	2	-.013	2	0	10	-1.758e-3	3	4791.119	2	1162.927	4
379		19	max	.006	3	.017	3	.046	4	6.701e-3	2	NC	1	NC	1
380			min	-.007	2	-.023	2	0	9	-3.604e-3	3	NC	1	1119.019	4
381	M5	1	max	.017	3	.066	3	.004	5	1.376e-5	4	NC	1	NC	1
382			min	-.02	2	-.056	2	0	9	5.819e-8	9	NC	1	NC	1
383		2	max	.017	3	.037	3	.006	5	1.011e-4	3	NC	4	NC	1
384			min	-.02	2	-.03	2	0	9	-6.658e-6	9	1639.285	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385	3	max	.017	3	.009	3	.008	5	1.936e-4	3	NC	5	NC	1
386		min	-.02	2	-.006	2	0	9	-1.323e-5	9	851.159	3	NC	1
387	4	max	.017	3	.015	2	.01	5	1.895e-4	3	NC	5	NC	1
388		min	-.02	2	-.014	3	0	9	-1.236e-5	9	609.53	3	NC	1
389	5	max	.017	3	.033	2	.012	5	1.854e-4	3	NC	5	NC	1
390		min	-.02	2	-.032	3	0	9	-1.148e-5	9	494.899	3	NC	1
391	6	max	.017	3	.049	2	.015	5	1.813e-4	3	NC	5	NC	1
392		min	-.02	2	-.047	3	0	9	-1.06e-5	9	429.799	2	NC	1
393	7	max	.017	3	.061	2	.018	5	1.792e-4	5	NC	5	NC	1
394		min	-.02	2	-.058	3	0	9	-9.723e-6	9	383.429	2	9700.517	3
395	8	max	.017	3	.07	2	.021	4	1.831e-4	5	NC	5	NC	1
396		min	-.02	2	-.066	3	0	9	-8.845e-6	9	354.44	2	9606.645	3
397	9	max	.017	3	.076	2	.024	4	1.87e-4	5	NC	5	NC	1
398		min	-.02	2	-.07	3	0	9	-7.967e-6	9	337.375	2	9837.42	3
399	10	max	.017	3	.078	2	.027	4	1.908e-4	5	NC	5	NC	1
400		min	-.02	2	-.07	3	0	9	-7.089e-6	9	329.636	2	NC	1
401	11	max	.017	3	.077	2	.03	4	1.948e-4	4	NC	5	NC	1
402		min	-.02	2	-.068	3	0	9	-6.212e-6	9	330.296	2	NC	1
403	12	max	.017	3	.072	2	.032	4	1.991e-4	4	NC	5	NC	1
404		min	-.02	2	-.062	3	0	9	-5.334e-6	9	339.752	2	NC	1
405	13	max	.017	3	.064	2	.035	4	2.033e-4	4	NC	5	NC	1
406		min	-.02	2	-.054	3	0	9	-4.456e-6	9	359.933	2	NC	1
407	14	max	.017	3	.051	2	.038	4	2.076e-4	4	NC	5	NC	1
408		min	-.02	2	-.042	3	0	9	-3.579e-6	9	395.266	2	NC	1
409	15	max	.017	3	.034	2	.04	4	2.119e-4	4	NC	5	NC	1
410		min	-.02	2	-.028	3	0	9	-2.701e-6	9	455.441	2	NC	1
411	16	max	.017	3	.014	2	.042	4	3.525e-4	4	NC	5	NC	1
412		min	-.02	2	-.012	3	0	9	-2.358e-6	9	564.034	2	NC	1
413	17	max	.017	3	.007	3	.044	4	3.736e-3	4	NC	5	NC	1
414		min	-.02	2	-.011	2	0	9	-1.473e-5	9	795.436	2	NC	1
415	18	max	.017	3	.028	3	.045	4	1.92e-3	4	NC	4	NC	1
416		min	-.02	2	-.041	2	0	9	-7.588e-6	9	1538.974	2	NC	1
417	19	max	.017	3	.05	3	.046	4	5.673e-6	5	NC	1	NC	1
418		min	-.02	2	-.072	2	0	9	-1.002e-6	3	NC	1	NC	1
419	M9	1	max	.006	.021	3	.004	5	6.844e-3	3	NC	1	NC	1
420		min	-.007	2	-.018	2	0	9	-4.788e-3	2	NC	1	NC	1
421	2	max	.006	3	.011	3	.004	4	3.396e-3	3	NC	4	NC	1
422		min	-.007	2	-.01	2	0	10	-2.369e-3	2	5053.425	3	NC	1
423	3	max	.006	3	.003	3	.004	4	5.789e-5	1	NC	4	NC	1
424		min	-.007	2	-.002	2	0	3	-2.834e-5	5	2622.359	3	NC	1
425	4	max	.006	3	.005	2	.004	4	4.472e-5	1	NC	4	NC	1
426		min	-.007	2	-.005	3	-.001	3	-3.711e-5	5	1875.247	3	NC	1
427	5	max	.006	3	.011	2	.005	4	3.155e-5	1	NC	4	NC	1
428		min	-.007	2	-.011	3	-.002	3	-4.588e-5	5	1520.268	3	NC	1
429	6	max	.006	3	.016	2	.007	4	1.839e-5	1	NC	4	NC	1
430		min	-.007	2	-.016	3	-.003	3	-5.465e-5	5	1323.355	3	9356.341	3
431	7	max	.006	3	.02	2	.009	4	5.217e-6	1	NC	4	NC	1
432		min	-.007	2	-.019	3	-.004	3	-6.55e-5	4	1194.359	2	8582.939	3
433	8	max	.006	3	.022	2	.011	4	8.172e-7	10	NC	4	NC	1
434		min	-.007	2	-.022	3	-.004	3	-7.708e-5	4	1104.268	2	6207.431	4
435	9	max	.006	3	.024	2	.014	4	2.404e-6	10	NC	4	NC	1
436		min	-.007	2	-.023	3	-.004	3	-8.866e-5	4	1051.241	2	4577.997	4
437	10	max	.006	3	.025	2	.017	4	3.992e-6	10	NC	4	NC	1
438		min	-.007	2	-.023	3	-.004	3	-1.002e-4	4	1027.21	2	3557.667	4
439	11	max	.006	3	.025	2	.02	5	5.579e-6	10	NC	4	NC	1
440		min	-.007	2	-.022	3	-.004	3	-1.118e-4	4	1029.3	2	2874.286	4
441	12	max	.006	3	.023	2	.024	5	7.166e-6	10	NC	4	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
442		min	-.007	2	-.021	3	-.004	3	-1.234e-4	4	1058.752	2	2391.256	5
443	13	max	.006	3	.02	2	.027	5	8.753e-6	10	NC	4	NC	1
444		min	-.007	2	-.018	3	-.004	3	-1.35e-4	4	1121.574	2	2034.987	5
445	14	max	.006	3	.016	2	.03	5	1.034e-5	10	NC	4	NC	1
446		min	-.007	2	-.014	3	-.003	3	-1.466e-4	4	1231.555	2	1769.804	5
447	15	max	.006	3	.011	2	.034	5	1.193e-5	10	NC	4	NC	1
448		min	-.007	2	-.009	3	-.003	3	-1.581e-4	4	1418.863	2	1567.737	5
449	16	max	.006	3	.004	2	.037	5	1.312e-5	10	NC	4	NC	1
450		min	-.007	2	-.004	3	-.002	3	-1.146e-4	3	1756.916	2	1411.054	5
451	17	max	.006	3	.002	3	.04	5	3.661e-3	4	NC	4	NC	1
452		min	-.007	2	-.004	2	-.001	3	-5.038e-5	9	2477.376	2	1287.934	5
453	18	max	.006	3	.009	3	.043	4	1.809e-3	5	NC	4	NC	1
454		min	-.007	2	-.013	2	0	9	-3.322e-3	2	4792.211	2	1186.846	4
455	19	max	.006	3	.017	3	.046	4	3.602e-3	3	NC	1	NC	1
456		min	-.007	2	-.023	2	0	9	-6.702e-3	2	NC	1	1105.53	4
457	M13	1	max	0	.021	3	.006	3	3.598e-3	3	NC	1	NC	1
458		min	-.004	5	-.018	2	-.007	2	-3.116e-3	2	NC	1	NC	1
459	2	max	0	9	.052	3	.004	3	4.437e-3	3	NC	4	NC	1
460		min	-.004	5	-.04	2	-.006	2	-3.846e-3	2	2717.377	3	NC	1
461	3	max	0	9	.078	3	.004	3	5.276e-3	3	NC	4	NC	1
462		min	-.004	5	-.058	2	-.007	2	-4.576e-3	2	1469.041	3	NC	1
463	4	max	0	9	.097	3	.005	9	6.115e-3	3	NC	4	NC	1
464		min	-.004	5	-.072	2	-.007	2	-5.306e-3	2	1110.784	3	NC	1
465	5	max	0	9	.106	3	.006	3	6.954e-3	3	NC	5	NC	1
466		min	-.004	5	-.079	2	-.009	2	-6.035e-3	2	992.291	3	NC	1
467	6	max	0	9	.105	3	.008	3	7.793e-3	3	NC	5	NC	1
468		min	-.004	5	-.079	2	-.011	2	-6.765e-3	2	996.663	3	NC	1
469	7	max	0	9	.097	3	.01	3	8.632e-3	3	NC	4	NC	1
470		min	-.004	5	-.075	2	-.014	2	-7.495e-3	2	1103.608	3	NC	1
471	8	max	0	9	.085	3	.013	3	9.471e-3	3	NC	4	NC	1
472		min	-.004	5	-.067	2	-.017	2	-8.225e-3	2	1325.913	3	8148.225	2
473	9	max	0	9	.072	3	.015	3	1.031e-2	3	NC	4	NC	1
474		min	-.004	5	-.059	2	-.019	2	-8.955e-3	2	1653.718	3	6654.032	2
475	10	max	0	9	.066	3	.017	3	1.115e-2	3	NC	4	NC	4
476		min	-.004	5	-.056	2	-.02	2	-9.685e-3	2	1873.593	3	6172.536	2
477	11	max	0	9	.072	3	.018	3	1.031e-2	3	NC	4	NC	1
478		min	-.004	5	-.059	2	-.019	2	-8.955e-3	2	1653.718	3	6654.052	2
479	12	max	0	9	.085	3	.018	3	9.474e-3	3	NC	4	NC	1
480		min	-.004	5	-.067	2	-.017	2	-8.225e-3	2	1325.913	3	6727.949	3
481	13	max	0	9	.097	3	.018	3	8.637e-3	3	NC	4	NC	1
482		min	-.004	5	-.075	2	-.014	2	-7.495e-3	2	1103.608	3	7116.453	3
483	14	max	0	9	.106	3	.016	3	7.8e-3	3	NC	5	NC	1
484		min	-.004	5	-.079	2	-.011	2	-6.765e-3	2	996.663	3	8096.059	3
485	15	max	0	9	.106	3	.014	3	6.963e-3	3	NC	5	NC	1
486		min	-.004	5	-.079	2	-.009	2	-6.035e-3	2	992.29	3	9993.916	3
487	16	max	0	9	.097	3	.012	3	6.126e-3	3	NC	4	NC	1
488		min	-.004	5	-.072	2	-.007	2	-5.306e-3	2	1110.784	3	NC	1
489	17	max	0	9	.079	3	.01	3	5.289e-3	3	NC	4	NC	1
490		min	-.004	5	-.058	2	-.007	2	-4.576e-3	2	1469.041	3	NC	1
491	18	max	0	9	.052	3	.008	3	4.451e-3	3	NC	4	NC	1
492		min	-.004	5	-.04	2	-.006	2	-3.846e-3	2	2717.376	3	NC	1
493	19	max	0	9	.022	3	.006	3	3.614e-3	3	NC	1	NC	1
494		min	-.005	5	-.018	2	-.007	2	-3.116e-3	2	NC	1	NC	1
495	M16	1	max	0	.017	3	.006	3	3.816e-3	2	NC	1	NC	1
496		min	-.046	4	-.023	2	-.007	2	-2.774e-3	3	NC	1	NC	1
497	2	max	0	9	.033	3	.008	3	4.712e-3	2	NC	4	NC	1
498		min	-.046	4	-.053	2	-.006	2	-3.39e-3	3	2769.508	2	NC	1



RISA-3D Version 13.0.0    \...\...\PVMMini 60 Cell 1V 20° 130mph 30psf 3.5ft 7-05.r    Page 41



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.004	4	-0.006	9	-0.033	3	-4.161e-3	2	8334.129	1	1196.166	3
557	13	max	0	3	.004	5	.025	1	4.965e-3	3	NC	3	9317.723	15
558		min	-0.004	4	-0.005	9	-0.032	3	-4.536e-3	2	9025.424	1	1183.512	3
559	14	max	0	3	.004	5	.023	1	5.349e-3	3	NC	1	NC	15
560		min	-0.005	4	-0.005	9	-.03	3	-4.91e-3	2	NC	1	1219.701	3
561	15	max	.001	3	.004	5	.02	1	5.733e-3	3	NC	1	NC	5
562		min	-0.005	4	-0.004	9	-.025	3	-5.285e-3	2	NC	1	1323.58	3
563	16	max	.001	3	.004	5	.014	1	6.117e-3	3	NC	1	NC	4
564		min	-0.005	4	-0.004	9	-.018	3	-5.659e-3	2	NC	1	1546.467	3
565	17	max	.001	3	.004	5	.007	1	6.501e-3	3	NC	1	NC	4
566		min	-0.006	4	-0.003	9	-.009	3	-6.034e-3	2	NC	1	2049.478	3
567	18	max	.001	3	.004	5	.004	3	6.885e-3	3	NC	1	NC	4
568		min	-0.006	4	-0.002	9	-.007	2	-6.409e-3	2	NC	1	3647.767	3
569	19	max	.001	3	.004	5	.02	3	7.269e-3	3	NC	1	NC	1
570		min	-0.007	4	0	9	-.021	2	-6.783e-3	2	NC	1	NC	1
571	M16A	1	max	0	.001	2	.006	3	2.127e-3	3	NC	1	NC	1
572		min	-0.002	4	-0.003	4	-.007	2	-2.17e-3	2	NC	1	NC	1
573	2	max	0	2	0	10	0	9	2.042e-3	3	NC	1	NC	1
574		min	-0.002	4	-0.005	4	-.002	2	-2.069e-3	2	NC	1	NC	1
575	3	max	0	2	-.001	10	.003	1	1.956e-3	3	NC	1	NC	4
576		min	-0.002	4	-0.007	4	-.004	3	-1.967e-3	2	NC	1	5685.713	3
577	4	max	0	2	-.002	12	.005	1	1.871e-3	3	NC	1	NC	4
578		min	-0.002	4	-0.009	4	-.007	3	-1.866e-3	2	8125.017	4	4321.42	3
579	5	max	0	2	-.002	12	.007	1	1.786e-3	3	NC	1	NC	6
580		min	-0.002	4	-0.011	4	-.009	3	-1.765e-3	2	6340.033	4	3729.21	3
581	6	max	0	2	-.003	12	.008	1	1.701e-3	3	NC	1	NC	9
582		min	-0.002	4	-0.012	4	-.011	3	-1.664e-3	2	5335.808	4	3469.253	3
583	7	max	0	2	-.003	12	.008	1	1.616e-3	3	NC	3	NC	9
584		min	-0.001	4	-0.013	4	-.013	5	-1.562e-3	2	4731.899	4	3403.579	3
585	8	max	0	2	-.003	12	.008	1	1.53e-3	3	NC	3	NC	9
586		min	-0.001	4	-0.014	4	-.015	5	-1.461e-3	2	4369.463	4	3484.78	3
587	9	max	0	2	-.003	12	.008	1	1.445e-3	3	NC	3	NC	9
588		min	-0.001	4	-0.014	4	-.017	5	-1.36e-3	2	4174.378	4	3294.669	5
589	10	max	0	2	-.004	12	.007	1	1.36e-3	3	NC	3	NC	9
590		min	-0.001	4	-0.014	4	-.018	5	-1.259e-3	2	4112.663	4	3089.236	5
591	11	max	0	2	-.003	12	.006	1	1.275e-3	3	NC	3	NC	9
592		min	0	4	-0.014	4	-.018	5	-1.157e-3	2	4174.378	4	3025.544	5
593	12	max	0	2	-.003	12	.005	1	1.189e-3	3	NC	3	NC	9
594		min	0	4	-0.013	4	-.018	5	-1.056e-3	2	4369.463	4	3092.579	5
595	13	max	0	2	-.003	12	.004	1	1.104e-3	3	NC	3	NC	1
596		min	0	4	-0.012	4	-.017	5	-9.55e-4	2	4731.899	4	3306.779	5
597	14	max	0	2	-.003	12	.003	1	1.019e-3	3	NC	1	NC	1
598		min	0	4	-0.011	4	-.015	5	-8.538e-4	2	5335.808	4	3721.817	5
599	15	max	0	2	-.002	12	.001	1	9.337e-4	3	NC	1	NC	1
600		min	0	4	-0.009	4	-.012	5	-7.525e-4	2	6340.033	4	4464.372	5
601	16	max	0	2	-.002	12	0	9	8.485e-4	3	NC	1	NC	1
602		min	0	4	-0.007	4	-.009	5	-6.513e-4	2	8125.017	4	5849.484	5
603	17	max	0	2	-.001	12	0	9	7.633e-4	3	NC	1	NC	1
604		min	0	4	-0.005	4	-.006	5	-5.501e-4	2	NC	1	8850.491	5
605	18	max	0	2	0	12	0	3	7.577e-4	4	NC	1	NC	1
606		min	0	4	-0.002	4	-.003	5	-4.488e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	8.137e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-3.476e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

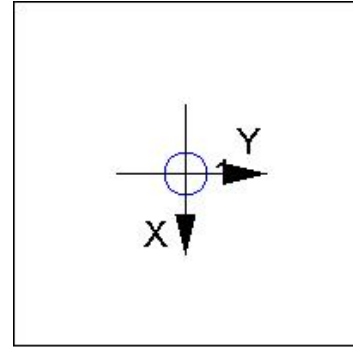
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
 Maximum concrete compression stress (psi): 0  
 Resultant tension force (lb): 405  
 Resultant compression force (lb): 0  
 Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
 Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
 Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
 Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

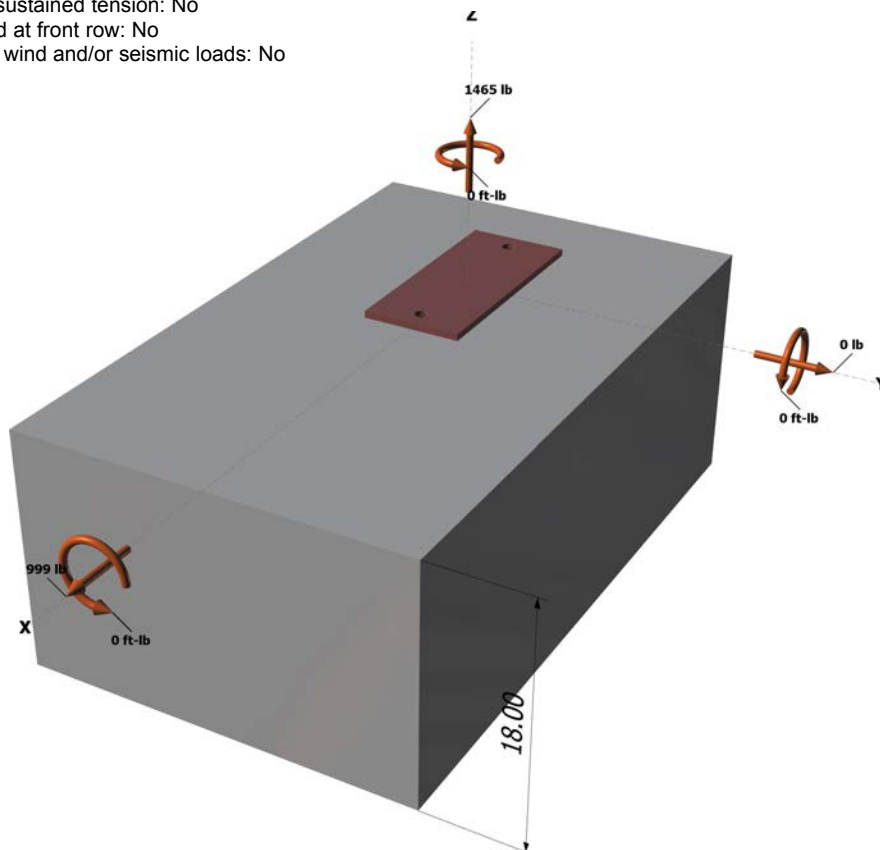
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

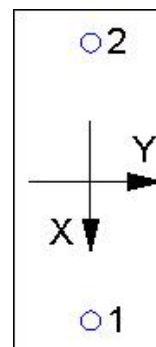
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{vc}$ (in <sup>2</sup> )	$A_{vco}$ (in <sup>2</sup> )	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

---

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

---

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.