

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	15° Tilt w/ Seismic Design
HCV		

## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1  
Module Tilt = 15°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	22.68 psf	(ASCE 7-05, Eq. 7-2)
$I_s$ =	1.00	
$C_s$ =	1.00	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	120 mph	Exposure Category = C
Height $\leq$	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 22.61 psf Including the gust factor,  $G=0.85$ . (ASCE 7-05, Eq. 6-15)

### Pressure Coefficients

$C_{f+ TOP}$ =	1	(Pressure)
$C_{f+ BOTTOM}$ =	1.6	
$C_{f- TOP}$ =	-2.04	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads

$S_S$ =	2.50	$R$ = 1.25
$S_{DS}$ =	1.67	$C_s$ = 0.8
$S_1$ =	1.00	$\rho$ = 1.3
$S_{D1}$ =	1.00	$\Omega$ = 1.25
$T_a$ =	0.04	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				



#### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	1.139 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	<b>9%</b>



#### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

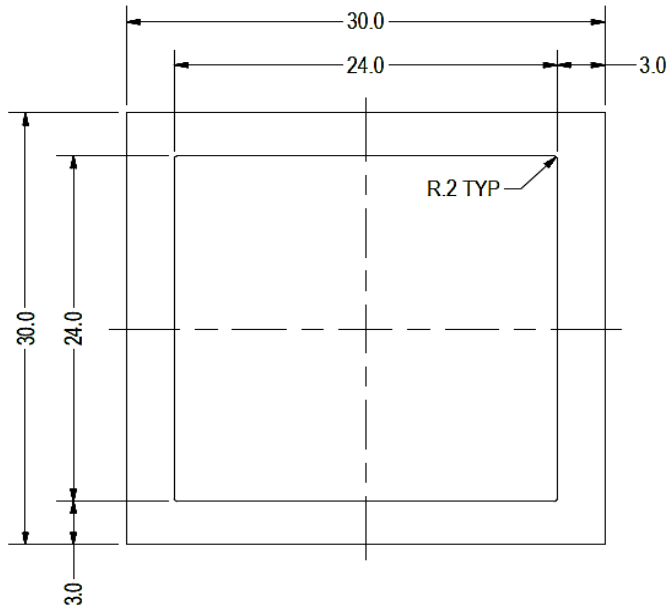
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.190 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	<b>5%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

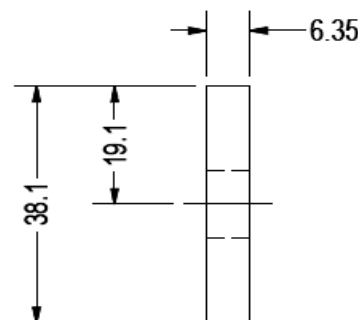
Strut Type =	<b>30x30x3</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	29.96 in
$\Phi F_{ty \text{ AXIAL}}$ =	16.11 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.52 ksi
$S_y$ =	0.16 in <sup>3</sup>
$S_x$ =	0.16 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.10 in <sup>4</sup>
$I_x$ =	0.10 in <sup>4</sup>
$A$ =	0.50 in <sup>2</sup>
$g$ =	0.60 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	0.869 k
$M_{y \text{ allowable}}$ =	0.413 k-ft
$M_{z \text{ allowable}}$ =	0.413 k-ft
$P_{n \text{ allowable}}$ =	8.089 k
Utilization =	<b>11%</b>



#### 4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	<b>1.5x0.25</b>
Aluminum Type =	6061-T6
$F_{ty}$ =	35 ksi
$\Phi$ =	0.90
$S_y$ =	0.02 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	33.25 in <sup>4</sup>
$A$ =	0.38 in <sup>2</sup>
$g$ =	0.45 lbs/ft
$M_y$ =	0.003 k-ft
$P_n$ =	0.169 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<b>8%</b>



A cross brace kit is required every 24 bays and is to be installed in centermost bays.

### 5. FOUNDATION DESIGN CALCULATIONS

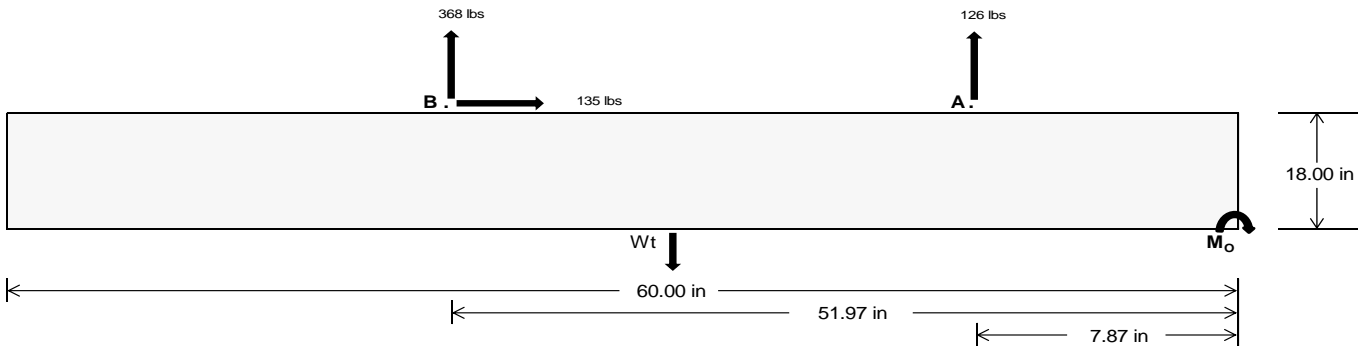
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>526.63</u>	<u>1532.12</u>	k
Compressive Load =	<u>1480.22</u>	<u>1066.86</u>	k
Lateral Load =	<u>24.22</u>	<u>560.58</u>	k
Moment (Weak Axis) =	<u>0.04</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 22528.3$  in-lbs  
Resisting Force Required = 750.94 lbs  
S.F. = 1.67  
Weight Required = 1251.57 lbs  
Minimum Width = 22 in  
Weight Provided = 1993.75 lbs

### Sliding

Force = 134.76 lbs  
Friction = 0.4  
Weight Required = 336.89 lbs  
Resisting Weight = 1993.75 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 134.76 lbs  
Cohesion = 130 psf  
Area = 9.17 ft<sup>2</sup>  
Resisting = 996.88 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$   
22 in 23 in 24 in 25 in  
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
$F_A$	471 lbs	471 lbs	471 lbs	471 lbs	580 lbs	580 lbs	580 lbs	580 lbs	756 lbs	756 lbs	756 lbs	756 lbs	-252 lbs	-252 lbs	-252 lbs	-252 lbs
$F_B$	341 lbs	341 lbs	341 lbs	341 lbs	417 lbs	417 lbs	417 lbs	417 lbs	544 lbs	544 lbs	544 lbs	544 lbs	-736 lbs	-736 lbs	-736 lbs	-736 lbs
$F_V$	26 lbs	26 lbs	26 lbs	26 lbs	235 lbs	235 lbs	235 lbs	235 lbs	194 lbs	194 lbs	194 lbs	194 lbs	-270 lbs	-270 lbs	-270 lbs	-270 lbs
$P_{total}$	2806 lbs	2897 lbs	2988 lbs	3078 lbs	2991 lbs	3081 lbs	3172 lbs	3263 lbs	3293 lbs	3384 lbs	3475 lbs	3565 lbs	209 lbs	264 lbs	318 lbs	372 lbs
$M$	283 lbs-ft	283 lbs-ft	283 lbs-ft	283 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	659 lbs-ft	689 lbs-ft	689 lbs-ft	689 lbs-ft	689 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft	478 lbs-ft
$e$	0.10 ft	0.10 ft	0.09 ft	0.09 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.29 ft	1.82 ft	1.50 ft	1.29 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
$f_{min}$	269.1 psf	266.8 psf	264.8 psf	262.9 psf	240.0 psf	239.0 psf	238.1 psf	237.3 psf	269.1 psf	266.9 psf	264.8 psf	262.9 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	343.2 psf	337.7 psf	332.7 psf	328.1 psf	412.5 psf	404.0 psf	396.2 psf	389.1 psf	449.4 psf	439.3 psf	430.1 psf	421.6 psf	357.9 psf	133.9 psf	106.5 psf	98.1 psf

Maximum Bearing Pressure = 449 psf  
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

## Seismic Design

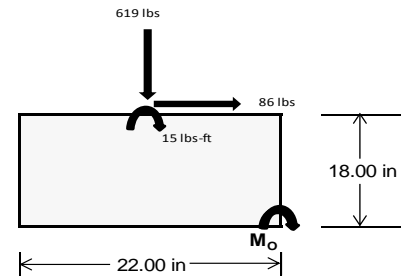
### Overturning Check

$M_o = 423.7 \text{ ft-lbs}$   
 Resisting Force Required = 462.19 lbs  
 S.F. = 1.67  
 Weight Required = 770.32 lbs  
 Minimum Width = 22 in in  
 Weight Provided = 1993.75 lbs

*A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.*

### Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	105 lbs	90 lbs	55 lbs	256 lbs	619 lbs	217 lbs	66 lbs	-12 lbs	18 lbs
$F_v$	14 lbs	114 lbs	14 lbs	10 lbs	86 lbs	10 lbs	14 lbs	114 lbs	14 lbs
$P_{total}$	2573 lbs	2558 lbs	2523 lbs	2605 lbs	2968 lbs	2567 lbs	788 lbs	710 lbs	740 lbs
$M$	38 lbs-ft	191 lbs-ft	39 lbs-ft	28 lbs-ft	144 lbs-ft	31 lbs-ft	38 lbs-ft	191 lbs-ft	39 lbs-ft
$e$	0.01 ft	0.07 ft	0.02 ft	0.01 ft	0.05 ft	0.01 ft	0.05 ft	0.27 ft	0.05 ft
$L/6$	0.31 ft	1.68 ft	1.80 ft	1.81 ft	1.74 ft	1.81 ft	1.74 ft	1.30 ft	1.73 ft
$f_{min}$	267.0 sqft	210.9 sqft	261.2 sqft	274.4 sqft	272.6 sqft	269.1 sqft	72.2 sqft	9.4 sqft	66.6 sqft
$f_{max}$	294.4 psf	347.1 psf	289.4 psf	294.1 psf	375.1 psf	290.9 psf	99.7 psf	145.6 psf	94.8 psf



Maximum Bearing Pressure = 375 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

### 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



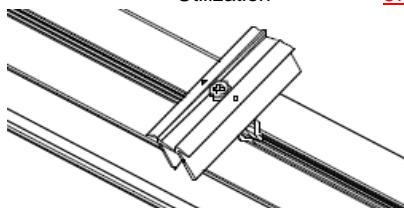
## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

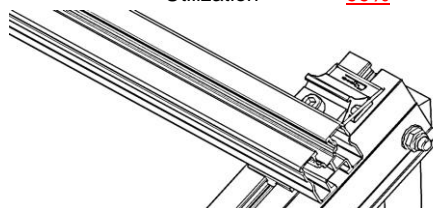
#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.694 k
Allowable Uplift =	1.214 k
Utilization =	<u>57%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	1.110 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



### 6.2 Bolted Connections

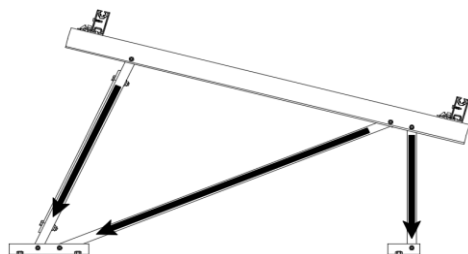
The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	1.139 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Diagonal Strut

Maximum Axial Load =	0.190 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>



#### Rear Strut

Maximum Axial Load =	1.166 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>20%</u>

#### Bracing

Maximum Axial Load =	0.169 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>2%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	28.39 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
	0.568 in
Max Drift, $\Delta_{MAX}$ =	0.061 in
	<u>0.061 ≤ 0.568. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus**

Strong Axis:

#### 3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$140.613$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.5 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 54.00 \text{ in}$$

$$J = 0.255$$

$$146.018$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.4$$

#### 3.4.16

$$b/t = 7.4$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 23.9$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \sqrt{b/t}]$$

$$\phi F_L = 28.5 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$\begin{aligned}
 h/t &= 23.9 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 30 \\
 Cc &= 30 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 29.5 \text{ ksi} \\
 I_x &= 250988 \text{ mm}^4 \\
 &= 0.603 \text{ in}^4 \\
 y &= 30 \text{ mm} \\
 S_x &= 0.511 \text{ in}^3 \\
 M_{\max} St &= 1.256 \text{ k-ft}
 \end{aligned}$$

### 3.4.18

$$\begin{aligned}
 h/t &= 7.4 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20 \\
 Cc &= 20 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 28.5 \text{ ksi} \\
 I_y &= 120291 \text{ mm}^4 \\
 &= 0.289 \text{ in}^4 \\
 x &= 20 \text{ mm} \\
 S_y &= 0.367 \text{ in}^3 \\
 M_{\max} Wk &= 0.871 \text{ k-ft}
 \end{aligned}$$

### Compression

#### 3.4.9

$$\begin{aligned}
 b/t &= 7.4 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 23.9 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= \phi c [Bp - 1.6Dp * b/t] \\
 \phi F_L &= 28.5 \text{ ksi}
 \end{aligned}$$

#### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 28.47 \text{ ksi} \\
 A &= 578.06 \text{ mm}^2 \\
 &= 0.90 \text{ in}^2 \\
 P_{\max} &= 25.51 \text{ kips}
 \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

### Strong Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &21.0529 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

N/A for Strong Direction

#### 3.4.16

$$\begin{aligned} b/t &= 4.29 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Strong Direction

### Weak Axis:

#### 3.4.11

$$\begin{aligned} L_b &= 33.78 \text{ in} \\ r_y &= 1.374 \\ C_b &= 1.36 \\ &24.5845 \\ S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\ S1 &= 1.37733 \\ S2 &= 1.2C_c \\ S2 &= 79.2 \\ \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{C_b})] \\ \phi F_L &= 29.8 \text{ ksi} \end{aligned}$$

#### 3.4.15

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\ S1 &= 3.8 \\ S2 &= \frac{k_1 Bp}{5.1Dp} \\ S2 &= 14.7 \\ F_{UT} &= (\phi b k_2 * \sqrt{BpE}) / (5.1b/t) \\ F_{UT} &= 9.4 \text{ ksi} \end{aligned}$$

#### 3.4.16

N/A for Weak Direction

#### 3.4.16

$$\begin{aligned} b/t &= 24.46 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\ F_{ST} &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.16.2

N/A for Strong Direction

### 3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

### 3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.463 \text{ k-ft}$$

### 3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

### 3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

### 3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} R_{b/t} &= 0.0 \\ S1 &= \left( \frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

#### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

## Compression

### 3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

### 3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$



#### A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

##### 3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

##### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

##### 3.4.16.1

N/A for Weak Direction

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

##### 3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.5 \text{ ksi}$$

Weak Axis:

**3.4.14**

$$L_b = 29.96 \text{ in}$$

$$J = 0.16$$

$$78.5957$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 30.5$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16**

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

**3.4.16.1** Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

**3.4.16.1**

N/A for Weak Direction

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

**3.4.18**

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.5 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.413 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 1.28467 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.75985 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 16.1143 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

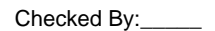
### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 16.11 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 8.09 \text{ kips}\end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



RISA-3D Version 13.0.0      \...\PVMMini 60 Cell 1V 15° 120mph 30psf 4.5ft 7-05.rdb      Page 20





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	272.708	1	.132	6	.199	1	0	10	0	4	0	15
30			min	-365.456	3	.029	15	-.401	5	0	4	0	3	0	6
31		16	max	272.805	1	.094	2	.199	1	0	10	0	4	0	15
32			min	-365.383	3	.02	15	-.488	5	0	4	0	3	0	6
33		17	max	272.901	1	.064	2	.199	1	0	10	0	1	0	15
34			min	-365.311	3	.011	15	-.575	5	0	4	0	3	0	6
35		18	max	272.998	1	.035	2	.199	1	0	10	0	1	0	15
36			min	-365.239	3	0	9	-.663	5	0	4	0	3	0	6
37		19	max	273.094	1	.008	10	.199	1	0	10	0	1	0	15
38			min	-365.167	3	-.027	1	-.75	5	0	4	0	3	0	6
39	M3	1	max	50.732	2	1.812	6	-.002	10	0	5	0	4	0	6
40			min	-55.49	9	.425	15	-1.357	4	0	1	0	10	0	15
41		2	max	50.664	2	1.634	6	-.002	10	0	5	0	1	0	6
42			min	-55.546	9	.383	15	-1.224	4	0	1	0	10	0	15
43		3	max	50.597	2	1.456	6	-.002	10	0	5	0	1	0	2
44			min	-55.602	9	.341	15	-1.09	4	0	1	0	10	0	15
45		4	max	50.53	2	1.278	6	-.002	10	0	5	0	1	0	15
46			min	-55.658	9	.299	15	-.957	4	0	1	0	5	0	4
47		5	max	50.463	2	1.1	6	-.002	10	0	5	0	1	0	15
48			min	-55.714	9	.257	15	-.823	4	0	1	0	5	0	4
49		6	max	50.396	2	.922	6	-.002	10	0	5	0	1	0	15
50			min	-55.77	9	.215	15	-.69	4	0	1	0	5	0	4
51		7	max	50.329	2	.744	6	-.002	10	0	5	0	1	0	15
52			min	-55.826	9	.174	15	-.556	4	0	1	0	5	0	4
53		8	max	50.262	2	.566	6	-.002	10	0	5	0	1	0	15
54			min	-55.882	9	.132	15	-.422	4	0	1	0	5	0	4
55		9	max	50.195	2	.388	6	-.002	10	0	5	0	1	0	15
56			min	-55.938	9	.09	15	-.289	4	0	1	0	5	-.001	4
57		10	max	50.128	2	.21	6	-.002	10	0	5	0	1	0	15
58			min	-55.994	9	.048	15	-.186	1	0	1	0	5	-.001	4
59		11	max	50.061	2	.038	2	.017	5	0	5	0	1	0	15
60			min	-56.049	9	.006	15	-.186	1	0	1	0	5	-.001	4
61		12	max	49.994	2	-.036	15	.15	5	0	5	0	1	0	15
62			min	-56.105	9	-.146	4	-.186	1	0	1	0	5	-.001	4
63		13	max	49.926	2	-.078	15	.284	5	0	5	0	1	0	15
64			min	-56.161	9	-.324	4	-.186	1	0	1	0	5	-.001	4
65		14	max	49.859	2	-.119	15	.417	5	0	5	0	9	0	15
66			min	-56.217	9	-.502	4	-.186	1	0	1	0	5	-.001	4
67		15	max	49.792	2	-.161	15	.551	5	0	5	0	10	0	15
68			min	-56.273	9	-.68	4	-.186	1	0	1	0	4	0	4
69		16	max	49.725	2	-.203	15	.684	5	0	5	0	10	0	15
70			min	-56.329	9	-.858	4	-.186	1	0	1	0	4	0	4
71		17	max	49.658	2	-.245	15	.818	5	0	5	0	10	0	15
72			min	-56.385	9	-1.037	4	-.186	1	0	1	0	4	0	4
73		18	max	49.591	2	-.287	15	.951	5	0	5	0	10	0	15
74			min	-56.441	9	-1.215	4	-.186	1	0	1	0	1	0	4
75		19	max	49.524	2	-.329	15	1.085	5	0	5	0	5	0	1
76			min	-56.497	9	-1.393	4	-.186	1	0	1	0	1	0	1
77	M4	1	max	376.03	1	0	1	.003	10	0	1	0	5	0	1
78			min	-119.298	3	0	1	-17.434	4	0	1	0	2	0	1
79		2	max	376.094	1	0	1	.003	10	0	1	0	12	0	1
80			min	-119.249	3	0	1	-17.491	4	0	1	-.002	4	0	1
81		3	max	376.159	1	0	1	.003	10	0	1	0	10	0	1
82			min	-119.201	3	0	1	-17.547	4	0	1	-.003	4	0	1
83		4	max	376.224	1	0	1	.003	10	0	1	0	10	0	1
84			min	-119.152	3	0	1	-17.603	4	0	1	-.005	4	0	1
85		5	max	376.288	1	0	1	.003	10	0	1	0	10	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
86			min	-119.104	3	0	1	-17.659	4	0	1	-.006	4	0	1
87		6	max	376.353	1	0	1	.003	10	0	1	0	10	0	1
88			min	-119.055	3	0	1	-17.715	4	0	1	-.008	4	0	1
89		7	max	376.418	1	0	1	.003	10	0	1	0	10	0	1
90			min	-119.007	3	0	1	-17.771	4	0	1	-.009	4	0	1
91		8	max	376.482	1	0	1	.003	10	0	1	0	10	0	1
92			min	-118.958	3	0	1	-17.827	4	0	1	-.011	4	0	1
93		9	max	376.547	1	0	1	.003	10	0	1	0	10	0	1
94			min	-118.91	3	0	1	-17.883	4	0	1	-.013	4	0	1
95		10	max	376.612	1	0	1	.003	10	0	1	0	10	0	1
96			min	-118.861	3	0	1	-17.939	4	0	1	-.014	4	0	1
97		11	max	376.677	1	0	1	.003	10	0	1	0	10	0	1
98			min	-118.813	3	0	1	-17.995	4	0	1	-.016	4	0	1
99		12	max	376.741	1	0	1	.003	10	0	1	0	10	0	1
100			min	-118.764	3	0	1	-18.051	4	0	1	-.017	4	0	1
101		13	max	376.806	1	0	1	.003	10	0	1	0	10	0	1
102			min	-118.715	3	0	1	-18.107	4	0	1	-.019	4	0	1
103		14	max	376.871	1	0	1	.003	10	0	1	0	10	0	1
104			min	-118.667	3	0	1	-18.163	4	0	1	-.021	4	0	1
105		15	max	376.935	1	0	1	.003	10	0	1	0	10	0	1
106			min	-118.618	3	0	1	-18.22	4	0	1	-.022	4	0	1
107		16	max	377	1	0	1	.003	10	0	1	0	10	0	1
108			min	-118.57	3	0	1	-18.276	4	0	1	-.024	4	0	1
109		17	max	377.065	1	0	1	.003	10	0	1	0	10	0	1
110			min	-118.521	3	0	1	-18.332	4	0	1	-.026	4	0	1
111		18	max	377.13	1	0	1	.003	10	0	1	0	10	0	1
112			min	-118.473	3	0	1	-18.388	4	0	1	-.027	4	0	1
113		19	max	377.194	1	0	1	.003	10	0	1	0	10	0	1
114			min	-118.424	3	0	1	-18.444	4	0	1	-.029	4	0	1
115	M6	1	max	867.291	1	.648	6	.866	4	0	3	0	3	0	1
116			min	-1166.497	3	.15	15	-.256	3	0	5	0	1	0	1
117		2	max	867.387	1	.61	6	.779	4	0	3	0	4	0	15
118			min	-1166.425	3	.141	15	-.256	3	0	5	0	2	0	6
119		3	max	867.483	1	.572	6	.692	4	0	3	0	4	0	15
120			min	-1166.353	3	.132	15	-.256	3	0	5	0	2	0	6
121		4	max	867.58	1	.534	6	.604	4	0	3	0	4	0	15
122			min	-1166.28	3	.123	15	-.256	3	0	5	0	2	0	6
123		5	max	867.676	1	.496	6	.517	4	0	3	0	4	0	15
124			min	-1166.208	3	.115	15	-.256	3	0	5	0	3	0	6
125		6	max	867.773	1	.459	6	.43	4	0	3	0	4	0	15
126			min	-1166.136	3	.106	15	-.256	3	0	5	0	3	0	6
127		7	max	867.869	1	.421	6	.342	4	0	3	0	4	0	15
128			min	-1166.064	3	.097	15	-.256	3	0	5	0	3	0	6
129		8	max	867.965	1	.383	6	.255	4	0	3	0	4	0	15
130			min	-1165.991	3	.088	15	-.256	3	0	5	0	3	0	6
131		9	max	868.062	1	.345	6	.168	4	0	3	0	4	0	15
132			min	-1165.919	3	.079	15	-.256	3	0	5	0	3	0	6
133		10	max	868.158	1	.307	6	.08	4	0	3	0	4	0	15
134			min	-1165.847	3	.07	15	-.256	3	0	5	0	3	0	6
135		11	max	868.254	1	.27	2	.055	9	0	3	0	4	0	15
136			min	-1165.774	3	.061	15	-.256	3	0	5	0	3	0	6
137		12	max	868.351	1	.241	2	.055	9	0	3	0	4	0	15
138			min	-1165.702	3	.052	15	-.256	3	0	5	0	3	0	6
139		13	max	868.447	1	.211	2	.055	9	0	3	0	4	0	15
140			min	-1165.63	3	.043	15	-.256	3	0	5	0	3	0	6
141		14	max	868.544	1	.182	2	.055	9	0	3	0	4	0	15
142			min	-1165.558	3	.035	15	-.29	5	0	5	0	3	0	6





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	868.64	1	.152	2	.055	9	0	3	0	4	0	15
144		min	-1165.485	3	.026	15	-.377	5	0	5	0	3	0	6
145	16	max	868.736	1	.123	2	.055	9	0	3	0	4	0	15
146		min	-1165.413	3	.017	15	-.464	5	0	5	0	3	0	6
147	17	max	868.833	1	.093	2	.055	9	0	3	0	4	0	15
148		min	-1165.341	3	-.003	9	-.552	5	0	5	0	3	0	6
149	18	max	868.929	1	.064	2	.055	9	0	3	0	4	0	15
150		min	-1165.269	3	-.028	9	-.639	5	0	5	0	3	0	6
151	19	max	869.025	1	.034	2	.055	9	0	3	0	4	0	15
152		min	-1165.196	3	-.052	9	-.726	5	0	5	0	3	0	6
153	M7	1	max	189.885	2	1.817	4	0	2	0	1	0	4	4
154		min	-100.03	9	.431	15	-1.42	4	0	3	0	3	0	15
155	2	max	189.818	2	1.639	4	0	2	0	1	0	4	0	2
156		min	-100.085	9	.389	15	-1.287	4	0	3	0	3	0	15
157	3	max	189.751	2	1.461	4	0	2	0	1	0	4	0	2
158		min	-100.141	9	.347	15	-1.153	4	0	3	0	3	0	9
159	4	max	189.684	2	1.283	4	0	2	0	1	0	1	0	10
160		min	-100.197	9	.305	15	-1.02	4	0	3	0	3	0	9
161	5	max	189.616	2	1.105	4	0	2	0	1	0	1	0	15
162		min	-100.253	9	.264	15	-.886	4	0	3	0	5	0	6
163	6	max	189.549	2	.927	4	0	2	0	1	0	1	0	15
164		min	-100.309	9	.222	15	-.753	4	0	3	0	5	0	6
165	7	max	189.482	2	.749	4	0	2	0	1	0	1	0	15
166		min	-100.365	9	.18	15	-.619	4	0	3	0	5	0	6
167	8	max	189.415	2	.571	4	0	2	0	1	0	1	0	15
168		min	-100.421	9	.138	15	-.486	4	0	3	0	5	0	6
169	9	max	189.348	2	.393	4	0	2	0	1	0	1	0	15
170		min	-100.477	9	.096	15	-.352	4	0	3	0	5	-.001	6
171	10	max	189.281	2	.215	4	0	2	0	1	0	1	0	15
172		min	-100.533	9	.054	15	-.219	4	0	3	0	5	-.001	6
173	11	max	189.214	2	.057	2	0	2	0	1	0	1	0	15
174		min	-100.589	9	-.001	9	-.085	4	0	3	0	5	-.001	6
175	12	max	189.147	2	-.029	15	.051	5	0	1	0	1	0	15
176		min	-100.645	9	-.141	6	-.012	1	0	3	0	5	-.001	6
177	13	max	189.08	2	-.071	15	.184	5	0	1	0	1	0	15
178		min	-100.7	9	-.319	6	-.012	1	0	3	0	5	-.001	6
179	14	max	189.013	2	-.113	15	.318	5	0	1	0	1	0	15
180		min	-100.756	9	-.497	6	-.012	1	0	3	0	5	-.001	6
181	15	max	188.945	2	-.155	15	.451	5	0	1	0	1	0	15
182		min	-100.812	9	-.675	6	-.012	1	0	3	0	5	0	6
183	16	max	188.878	2	-.197	15	.585	5	0	1	0	1	0	15
184		min	-100.868	9	-.853	6	-.012	1	0	3	0	5	0	6
185	17	max	188.811	2	-.239	15	.718	5	0	1	0	1	0	15
186		min	-100.924	9	-1.031	6	-.012	1	0	3	0	5	0	6
187	18	max	188.744	2	-.28	15	.852	5	0	1	0	1	0	15
188		min	-100.98	9	-1.209	6	-.012	1	0	3	0	5	0	6
189	19	max	188.677	2	-.322	15	.985	5	0	1	0	1	0	1
190		min	-101.036	9	-1.387	6	-.012	1	0	3	0	3	0	1
191	M8	1	max	1137.466	1	0	.161	1	0	1	0	4	0	1
192		min	-405.971	3	0	1	-17.81	4	0	1	0	1	0	1
193	2	max	1137.531	1	0	1	.161	1	0	1	0	1	0	1
194		min	-405.922	3	0	1	-17.866	4	0	1	-.002	4	0	1
195	3	max	1137.596	1	0	1	.161	1	0	1	0	1	0	1
196		min	-405.874	3	0	1	-17.922	4	0	1	-.003	4	0	1
197	4	max	1137.661	1	0	1	.161	1	0	1	0	1	0	1
198		min	-405.825	3	0	1	-17.978	4	0	1	-.005	4	0	1
199	5	max	1137.725	1	0	1	.161	1	0	1	0	1	0	1



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-405.777	3	0	1	-18.035	4	0	1	-.006	4	0	1
201		6	max	1137.79	1	0	1	.161	1	0	1	0	1	0	1
202			min	-405.728	3	0	1	-18.091	4	0	1	-.008	4	0	1
203		7	max	1137.855	1	0	1	.161	1	0	1	0	1	0	1
204			min	-405.679	3	0	1	-18.147	4	0	1	-.01	4	0	1
205		8	max	1137.919	1	0	1	.161	1	0	1	0	1	0	1
206			min	-405.631	3	0	1	-18.203	4	0	1	-.011	4	0	1
207		9	max	1137.984	1	0	1	.161	1	0	1	0	1	0	1
208			min	-405.582	3	0	1	-18.259	4	0	1	-.013	4	0	1
209		10	max	1138.049	1	0	1	.161	1	0	1	0	1	0	1
210			min	-405.534	3	0	1	-18.315	4	0	1	-.015	4	0	1
211		11	max	1138.114	1	0	1	.161	1	0	1	0	1	0	1
212			min	-405.485	3	0	1	-18.371	4	0	1	-.016	4	0	1
213		12	max	1138.178	1	0	1	.161	1	0	1	0	1	0	1
214			min	-405.437	3	0	1	-18.427	4	0	1	-.018	4	0	1
215		13	max	1138.243	1	0	1	.161	1	0	1	0	1	0	1
216			min	-405.388	3	0	1	-18.483	4	0	1	-.019	4	0	1
217		14	max	1138.308	1	0	1	.161	1	0	1	0	1	0	1
218			min	-405.34	3	0	1	-18.539	4	0	1	-.021	4	0	1
219		15	max	1138.372	1	0	1	.161	1	0	1	0	1	0	1
220			min	-405.291	3	0	1	-18.595	4	0	1	-.023	4	0	1
221		16	max	1138.437	1	0	1	.161	1	0	1	0	1	0	1
222			min	-405.243	3	0	1	-18.651	4	0	1	-.024	4	0	1
223		17	max	1138.502	1	0	1	.161	1	0	1	0	1	0	1
224			min	-405.194	3	0	1	-18.707	4	0	1	-.026	4	0	1
225		18	max	1138.567	1	0	1	.161	1	0	1	0	1	0	1
226			min	-405.146	3	0	1	-18.764	4	0	1	-.028	4	0	1
227		19	max	1138.631	1	0	1	.161	1	0	1	0	1	0	1
228			min	-405.097	3	0	1	-18.82	4	0	1	-.029	4	0	1
229	M10	1	max	272.991	1	.69	4	1.011	5	0	1	0	4	0	1
230			min	-336.832	3	.173	15	-.098	1	-.001	5	0	3	0	1
231		2	max	273.087	1	.652	4	.924	5	0	1	0	4	0	15
232			min	-336.759	3	.164	15	-.098	1	-.001	5	0	3	0	4
233		3	max	273.184	1	.615	4	.836	5	0	1	0	4	0	15
234			min	-336.687	3	.155	15	-.098	1	-.001	5	0	3	0	4
235		4	max	273.28	1	.577	4	.749	5	0	1	0	4	0	15
236			min	-336.615	3	.146	15	-.098	1	-.001	5	0	3	0	4
237		5	max	273.376	1	.539	4	.662	5	0	1	0	5	0	15
238			min	-336.542	3	.137	15	-.098	1	-.001	5	0	3	0	4
239		6	max	273.473	1	.501	4	.574	5	0	1	0	5	0	15
240			min	-336.47	3	.129	15	-.098	1	-.001	5	0	3	0	4
241		7	max	273.569	1	.463	4	.487	5	0	1	0	5	0	15
242			min	-336.398	3	.12	15	-.098	1	-.001	5	0	3	0	4
243		8	max	273.665	1	.425	4	.4	5	0	1	0	5	0	15
244			min	-336.326	3	.111	15	-.098	1	-.001	5	0	3	0	4
245		9	max	273.762	1	.388	4	.312	5	0	1	0	5	0	15
246			min	-336.253	3	.102	15	-.098	1	-.001	5	0	3	0	4
247		10	max	273.858	1	.35	4	.225	5	0	1	0	5	0	15
248			min	-336.181	3	.093	15	-.098	1	-.001	5	0	3	0	4
249		11	max	273.955	1	.312	4	.138	5	0	1	0	5	0	15
250			min	-336.109	3	.084	15	-.098	1	-.001	5	0	3	0	4
251		12	max	274.051	1	.274	4	.05	5	0	1	0	5	0	15
252			min	-336.037	3	.075	15	-.098	1	-.001	5	0	3	0	4
253		13	max	274.147	1	.236	4	-.005	10	0	1	0	5	0	15
254			min	-335.964	3	.066	15	-.098	1	-.001	5	0	3	0	4
255		14	max	274.244	1	.198	4	-.005	10	0	1	0	5	0	15
256			min	-335.892	3	.057	15	-.133	4	-.001	5	0	3	0	4



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257	15	max	274.34	1	.161	4	-.005	10	0	1	0	5	0	15
258		min	-335.82	3	.049	15	-.22	4	-.001	5	0	3	0	4
259	16	max	274.436	1	.123	4	-.005	10	0	1	0	5	0	15
260		min	-335.747	3	.04	15	-.308	4	-.001	5	0	3	0	4
261	17	max	274.533	1	.085	4	-.005	10	0	1	0	5	0	15
262		min	-335.675	3	.024	9	-.395	4	-.001	5	0	3	0	4
263	18	max	274.629	1	.056	3	-.005	10	0	1	0	5	0	15
264		min	-335.603	3	-.001	9	-.482	4	-.001	5	0	3	0	4
265	19	max	274.725	1	.034	3	-.005	10	0	1	0	5	0	15
266		min	-335.531	3	-.027	1	-.57	4	-.001	5	0	3	0	4
267	M11	1	max	50.288	2	1.811	.203	1	0	4	.001	5	0	6
268		min	-55.573	9	.424	15	-1.222	5	0	10	0	1	0	15
269	2	max	50.221	2	1.633	6	.203	1	0	4	0	5	0	6
270		min	-55.629	9	.382	15	-1.089	5	0	10	0	1	0	15
271	3	max	50.154	2	1.455	6	.203	1	0	4	0	5	0	2
272		min	-55.685	9	.34	15	-.955	5	0	10	0	1	0	3
273	4	max	50.087	2	1.277	6	.203	1	0	4	0	5	0	15
274		min	-55.741	9	.298	15	-.822	5	0	10	0	1	0	4
275	5	max	50.02	2	1.099	6	.203	1	0	4	0	3	0	15
276		min	-55.797	9	.257	15	-.688	5	0	10	0	1	0	4
277	6	max	49.953	2	.921	6	.203	1	0	4	0	3	0	15
278		min	-55.853	9	.215	15	-.554	5	0	10	0	1	0	4
279	7	max	49.886	2	.743	6	.203	1	0	4	0	3	0	15
280		min	-55.909	9	.173	15	-.421	5	0	10	0	1	0	4
281	8	max	49.819	2	.565	6	.203	1	0	4	0	3	0	15
282		min	-55.965	9	.131	15	-.287	5	0	10	0	1	0	4
283	9	max	49.752	2	.387	6	.203	1	0	4	0	3	0	15
284		min	-56.021	9	.089	15	-.154	5	0	10	0	1	-.001	4
285	10	max	49.684	2	.209	6	.203	1	0	4	0	3	0	15
286		min	-56.077	9	.047	15	-.02	5	0	10	0	4	-.001	4
287	11	max	49.617	2	.038	2	.203	1	0	4	0	3	0	15
288		min	-56.133	9	0	3	-.017	3	0	10	0	4	-.001	4
289	12	max	49.55	2	-.036	15	.289	4	0	4	0	3	0	15
290		min	-56.188	9	-.147	4	-.017	3	0	10	0	4	-.001	4
291	13	max	49.483	2	-.078	15	.422	4	0	4	0	3	0	15
292		min	-56.244	9	-.325	4	-.017	3	0	10	0	5	-.001	4
293	14	max	49.416	2	-.12	15	.556	4	0	4	0	3	0	15
294		min	-56.3	9	-.503	4	-.017	3	0	10	0	10	-.001	4
295	15	max	49.349	2	-.162	15	.69	4	0	4	0	3	0	15
296		min	-56.356	9	-.681	4	-.017	3	0	10	0	10	0	4
297	16	max	49.282	2	-.204	15	.823	4	0	4	0	4	0	15
298		min	-56.412	9	-.859	4	-.017	3	0	10	0	10	0	4
299	17	max	49.215	2	-.246	15	.957	4	0	4	0	4	0	15
300		min	-56.468	9	-1.037	4	-.017	3	0	10	0	10	0	4
301	18	max	49.148	2	-.287	15	1.09	4	0	4	0	4	0	15
302		min	-56.524	9	-1.215	4	-.017	3	0	10	0	10	0	4
303	19	max	49.081	2	-.329	15	1.224	4	0	4	.001	4	0	1
304		min	-56.58	9	-1.393	4	-.017	3	0	10	0	10	0	1
305	M12	1	max	376.158	1	0	.778	1	0	1	0	4	0	1
306		min	-118.938	3	0	1	-16.331	5	0	1	0	3	0	1
307	2	max	376.223	1	0	1	.778	1	0	1	0	1	0	1
308		min	-118.889	3	0	1	-16.388	5	0	1	-.001	5	0	1
309	3	max	376.288	1	0	1	.778	1	0	1	0	1	0	1
310		min	-118.841	3	0	1	-16.444	5	0	1	-.003	5	0	1
311	4	max	376.353	1	0	1	.778	1	0	1	0	1	0	1
312		min	-118.792	3	0	1	-16.5	5	0	1	-.004	5	0	1
313	5	max	376.417	1	0	1	.778	1	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
314		min	-118.744	3	0	1	-16.556	5	0	1	-.006	5	0	1
315	6	max	376.482	1	0	1	.778	1	0	1	0	1	0	1
316		min	-118.695	3	0	1	-16.612	5	0	1	-.007	5	0	1
317	7	max	376.547	1	0	1	.778	1	0	1	0	1	0	1
318		min	-118.647	3	0	1	-16.668	5	0	1	-.009	5	0	1
319	8	max	376.611	1	0	1	.778	1	0	1	0	1	0	1
320		min	-118.598	3	0	1	-16.724	5	0	1	-.01	5	0	1
321	9	max	376.676	1	0	1	.778	1	0	1	0	1	0	1
322		min	-118.55	3	0	1	-16.78	5	0	1	-.012	5	0	1
323	10	max	376.741	1	0	1	.778	1	0	1	0	1	0	1
324		min	-118.501	3	0	1	-16.836	5	0	1	-.013	5	0	1
325	11	max	376.806	1	0	1	.778	1	0	1	0	1	0	1
326		min	-118.452	3	0	1	-16.892	5	0	1	-.015	5	0	1
327	12	max	376.87	1	0	1	.778	1	0	1	0	1	0	1
328		min	-118.404	3	0	1	-16.948	5	0	1	-.016	5	0	1
329	13	max	376.935	1	0	1	.778	1	0	1	0	1	0	1
330		min	-118.355	3	0	1	-17.004	5	0	1	-.018	5	0	1
331	14	max	377	1	0	1	.778	1	0	1	0	1	0	1
332		min	-118.307	3	0	1	-17.061	5	0	1	-.019	5	0	1
333	15	max	377.064	1	0	1	.778	1	0	1	0	1	0	1
334		min	-118.258	3	0	1	-17.117	5	0	1	-.021	5	0	1
335	16	max	377.129	1	0	1	.778	1	0	1	.001	1	0	1
336		min	-118.21	3	0	1	-17.173	5	0	1	-.022	5	0	1
337	17	max	377.194	1	0	1	.778	1	0	1	.001	1	0	1
338		min	-118.161	3	0	1	-17.229	5	0	1	-.024	5	0	1
339	18	max	377.259	1	0	1	.778	1	0	1	.001	1	0	1
340		min	-118.113	3	0	1	-17.285	5	0	1	-.026	5	0	1
341	19	max	377.323	1	0	1	.778	1	0	1	.001	1	0	1
342		min	-118.064	3	0	1	-17.341	5	0	1	-.027	5	0	1
343	M1	1	max	59.716	1	344.8	3	-.178	10	0	.033	1	0	1
344		min	3.696	12	-273.939	1	-16.993	1	0	3	0	10	0	3
345	2	max	59.788	1	344.598	3	-.178	10	0	1	.03	1	.06	1
346		min	3.732	12	-274.209	1	-16.993	1	0	3	0	10	-.075	3
347	3	max	69.838	1	4.675	14	-.174	10	0	5	.026	1	.118	1
348		min	-6.663	3	-21.881	3	-16.854	1	0	1	0	10	-.148	3
349	4	max	69.91	1	4.41	14	-.174	10	0	5	.022	1	.119	1
350		min	-6.609	3	-22.083	3	-16.854	1	0	1	0	10	-.144	3
351	5	max	69.983	1	4.145	14	-.174	10	0	5	.018	1	.12	1
352		min	-6.555	3	-22.285	3	-16.854	1	0	1	0	10	-.139	3
353	6	max	70.055	1	3.88	14	-.174	10	0	5	.015	1	.12	1
354		min	-6.5	3	-22.487	3	-16.854	1	0	1	0	10	-.134	3
355	7	max	70.127	1	3.615	14	-.174	10	0	5	.011	1	.121	1
356		min	-6.446	3	-22.69	3	-16.854	1	0	1	0	10	-.129	3
357	8	max	70.199	1	3.35	14	-.174	10	0	5	.007	1	.123	2
358		min	-6.392	3	-22.892	3	-16.854	1	0	1	0	10	-.124	3
359	9	max	70.272	1	3.085	14	-.174	10	0	5	.004	1	.127	2
360		min	-6.338	3	-23.094	3	-16.854	1	0	1	0	10	-.119	3
361	10	max	70.344	1	2.844	9	-.174	10	0	5	.001	3	.13	2
362		min	-6.284	3	-23.297	3	-16.854	1	0	1	0	10	-.114	3
363	11	max	70.416	1	2.62	9	-.174	10	0	5	0	3	.134	2
364		min	-6.229	3	-23.499	3	-16.854	1	0	1	-.004	1	-.109	3
365	12	max	70.489	1	2.395	9	-.174	10	0	5	0	12	.138	2
366		min	-6.175	3	-23.701	3	-16.854	1	0	1	-.007	1	-.104	3
367	13	max	70.561	1	2.17	9	-.174	10	0	5	0	10	.142	2
368		min	-6.121	3	-23.903	3	-16.854	1	0	1	-.011	1	-.099	3
369	14	max	70.633	1	1.945	9	-.174	10	0	5	0	10	.145	2
370		min	-6.067	3	-24.106	3	-16.854	1	0	1	-.014	1	-.093	3



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	70.705	1	1.721	9	-1.174	10	0	5	0	10	.149	2
372			min	-6.013	3	-24.308	3	-16.854	1	0	1	-.018	1	-.088	3
373		16	max	68.986	2	15.303	10	-1.176	10	0	1	0	10	.153	2
374			min	-34.641	3	-50.262	3	-17.027	1	0	4	-.022	1	-.083	3
375		17	max	69.059	2	15.079	10	-1.176	10	0	1	0	10	.15	2
376			min	-34.587	3	-50.465	3	-17.027	1	0	4	-.026	1	-.072	3
377		18	max	-2.419	12	348.965	2	-1.173	10	0	3	0	10	.076	2
378			min	-59.746	1	-165.979	3	-27.44	4	0	2	-.03	1	-.036	3
379		19	max	-2.383	12	348.696	2	-1.173	10	0	3	0	10	0	2
380			min	-59.674	1	-166.182	3	-27.198	4	0	2	-.033	1	0	3
381	M5	1	max	142.8	1	1119.426	3	0	10	0	9	.032	4	0	3
382			min	.212	15	-886.309	1	-46.617	3	0	5	0	10	0	1
383		2	max	142.872	1	1119.224	3	0	10	0	9	.028	4	.192	1
384			min	.234	15	-886.578	1	-46.617	3	0	5	-.004	3	-.242	3
385		3	max	173.772	1	6.727	9	5.109	3	0	3	.023	4	.381	1
386			min	-40.919	3	-76.809	3	-17.655	4	0	4	-.013	3	-.48	3
387		4	max	173.844	1	6.502	9	5.109	3	0	3	.019	4	.385	1
388			min	-40.865	3	-77.011	3	-17.413	4	0	4	-.012	3	-.463	3
389		5	max	173.916	1	6.278	9	5.109	3	0	3	.015	4	.389	1
390			min	-40.811	3	-77.213	3	-17.171	4	0	4	-.011	3	-.446	3
391		6	max	173.988	1	6.053	9	5.109	3	0	3	.012	4	.394	1
392			min	-40.757	3	-77.416	3	-16.929	4	0	4	-.01	3	-.43	3
393		7	max	174.061	1	5.828	9	5.109	3	0	3	.008	4	.398	1
394			min	-40.702	3	-77.618	3	-16.687	4	0	4	-.009	3	-.413	3
395		8	max	174.133	1	5.603	9	5.109	3	0	3	.004	4	.403	1
396			min	-40.648	3	-77.82	3	-16.445	4	0	4	-.008	3	-.396	3
397		9	max	174.205	1	5.378	9	5.109	3	0	3	0	4	.413	2
398			min	-40.594	3	-78.023	3	-16.203	4	0	4	-.007	3	-.379	3
399		10	max	174.277	1	5.154	9	5.109	3	0	3	0	2	.425	2
400			min	-40.54	3	-78.225	3	-15.961	4	0	4	-.006	3	-.362	3
401		11	max	174.35	1	4.929	9	5.109	3	0	3	0	2	.437	2
402			min	-40.486	3	-78.427	3	-15.719	4	0	4	-.006	4	-.345	3
403		12	max	174.422	1	4.704	9	5.109	3	0	3	0	2	.45	2
404			min	-40.431	3	-78.629	3	-15.477	4	0	4	-.009	4	-.328	3
405		13	max	174.494	1	4.479	9	5.109	3	0	3	0	2	.462	2
406			min	-40.377	3	-78.832	3	-15.235	4	0	4	-.013	4	-.311	3
407		14	max	174.567	1	4.255	9	5.109	3	0	3	0	2	.475	2
408			min	-40.323	3	-79.034	3	-14.993	4	0	4	-.016	4	-.294	3
409		15	max	174.639	1	4.03	9	5.109	3	0	3	0	3	.487	2
410			min	-40.269	3	-79.236	3	-14.751	4	0	4	-.019	4	-.277	3
411		16	max	228.615	2	66.559	2	5.083	3	0	3	0	3	.499	2
412			min	-108.584	3	-138.67	3	-13.53	4	0	4	-.023	4	-.259	3
413		17	max	228.687	2	66.289	2	5.083	3	0	3	.002	3	.484	2
414			min	-108.53	3	-138.872	3	-13.288	4	0	4	-.026	4	-.229	3
415		18	max	-2.579	12	1128.101	2	4.685	3	0	4	.003	3	.244	2
416			min	-142.964	1	-532.072	3	-30.448	5	0	1	-.032	4	-.115	3
417		19	max	-2.542	12	1127.832	2	4.685	3	0	4	.004	3	0	3
418			min	-142.891	1	-532.274	3	-30.206	5	0	1	-.039	4	0	2
419	M9	1	max	59.604	1	344.758	3	124.86	4	0	3	0	5	0	1
420			min	-.24	5	-273.938	1	.178	10	0	1	-.033	1	0	3
421		2	max	59.676	1	344.556	3	125.102	4	0	3	.027	5	.06	1
422			min	-.212	15	-274.208	1	.178	10	0	1	-.029	1	-.075	3
423		3	max	70.094	1	4.404	9	16.569	1	0	4	.052	5	.118	1
424			min	-6.779	3	-21.802	3	-23.384	5	0	10	-.025	1	-.148	3
425		4	max	70.167	1	4.179	9	16.569	1	0	4	.047	5	.119	1
426			min	-6.725	3	-22.004	3	-23.142	5	0	10	-.022	1	-.143	3
427		5	max	70.239	1	3.954	9	16.569	1	0	4	.042	5	.119	1







Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485	15	max	35.897	4	44.518	1	9.805	4	0	3	.001	5	.288	3
486		min	.178	10	-57.204	3	-1.09	2	0	1	-.028	1	-.229	1
487	16	max	30.986	4	43.292	3	16.026	1	0	3	.006	5	.291	3
488		min	.178	10	-35.05	1	.173	10	0	1	-.023	1	-.232	1
489	17	max	26.075	4	143.789	3	30.588	1	0	3	.011	5	.244	3
490		min	.178	10	-114.617	1	1.395	10	0	1	-.012	1	-.194	1
491	18	max	21.165	4	244.285	3	45.151	1	0	3	.019	4	.147	3
492		min	.178	10	-194.185	1	2.617	10	0	1	-.001	10	-.117	1
493	19	max	17.018	1	344.782	3	59.713	1	0	3	.033	1	0	1
494		min	.178	10	-273.752	1	3.696	12	0	1	0	10	0	3
495	M16	1	max	34.637	5	348.775	2	8.364	5	0	.033	1	0	2
496		min	-17.496	1	-166.19	3	-59.558	1	0	2	-.028	5	0	3
497	2	max	29.726	5	247.323	2	9.049	5	0	3	.007	1	.071	3
498		min	-17.496	1	-118.152	3	-44.995	1	0	2	-.023	5	-.149	2
499	3	max	24.816	5	145.871	2	9.733	5	0	3	0	12	.118	3
500		min	-17.496	1	-70.114	3	-30.433	1	0	2	-.021	4	-.247	2
501	4	max	19.905	5	44.419	2	10.417	5	0	3	-.001	12	.141	3
502		min	-17.496	1	-22.075	3	-15.871	1	0	2	-.024	1	-.295	2
503	5	max	14.994	5	25.963	3	11.102	5	0	3	-.002	12	.14	3
504		min	-17.496	1	-57.033	2	-1.727	3	0	2	-.028	1	-.292	2
505	6	max	10.083	5	74.001	3	14.236	4	0	3	-.002	15	.115	3
506		min	-17.496	1	-158.485	2	-1.083	3	0	2	-.025	1	-.238	2
507	7	max	5.172	5	122.039	3	27.816	1	0	3	.004	5	.066	3
508		min	-17.496	1	-259.936	2	-.439	3	0	2	-.015	1	-.133	2
509	8	max	1.837	3	170.077	3	42.378	1	0	3	.011	4	.022	2
510		min	-17.496	1	-361.388	2	.204	12	0	2	-.004	3	-.007	3
511	9	max	1.837	3	218.115	3	56.94	1	0	3	.028	1	.228	2
512		min	-17.496	1	-462.84	2	.633	12	0	2	-.004	3	-.104	3
513	10	max	20.652	5	-9.171	15	71.503	1	0	14	.06	1	.485	2
514		min	-17.496	1	-564.292	2	-2.164	3	0	2	-.003	3	-.225	3
515	11	max	15.741	5	462.84	2	5.821	5	0	2	.028	1	.228	2
516		min	-17.454	1	-218.115	3	-56.82	1	0	3	-.012	5	-.104	3
517	12	max	10.83	5	361.388	2	6.505	5	0	2	.003	2	.022	2
518		min	-17.454	1	-170.077	3	-42.258	1	0	3	-.009	5	-.007	3
519	13	max	5.919	5	259.936	2	7.189	5	0	2	0	10	.066	3
520		min	-17.454	1	-122.039	3	-27.696	1	0	3	-.015	1	-.133	2
521	14	max	1.009	5	158.485	2	7.874	5	0	2	0	12	.115	3
522		min	-17.454	1	-74.001	3	-13.133	1	0	3	-.025	1	-.238	2
523	15	max	-.173	10	57.033	2	9.094	4	0	2	.003	5	.14	3
524		min	-17.454	1	-25.963	3	-1.115	2	0	3	-.028	1	-.292	2
525	16	max	-.173	10	22.075	3	15.991	1	0	2	.007	5	.141	3
526		min	-17.454	1	-44.419	2	.158	10	0	3	-.023	1	-.295	2
527	17	max	-.173	10	70.114	3	30.553	1	0	2	.012	5	.118	3
528		min	-17.454	1	-145.871	2	1.38	10	0	3	-.012	1	-.247	2
529	18	max	-.173	10	118.152	3	45.116	1	0	2	.019	4	.071	3
530		min	-22.309	4	-247.323	2	1.953	12	0	3	-.001	10	-.149	2
531	19	max	-.173	10	166.19	3	59.678	1	0	2	.033	1	0	2
532		min	-27.219	4	-348.775	2	2.382	12	0	3	0	10	0	3
533	M15	1	max	0	.923	3	.107	3	0	1	0	1	0	1
534		min	-59.178	3	0	1	0	1	0	3	0	3	0	1
535	2	max	0	1	.821	3	.107	3	0	1	0	1	0	1
536		min	-59.232	3	0	1	0	1	0	3	0	3	0	3
537	3	max	0	1	.718	3	.107	3	0	1	0	1	0	1
538		min	-59.286	3	0	1	0	1	0	3	0	3	0	3
539	4	max	0	1	.616	3	.107	3	0	1	0	1	0	1
540		min	-59.34	3	0	1	0	1	0	3	0	3	0	3
541	5	max	0	1	.513	3	.107	3	0	1	0	1	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-59.394	3	0	1	0	1	0	3	0	3	0	3
543		6	max	0	1	.41	3	.107	3	0	1	0	1	0	1
544			min	-59.448	3	0	1	0	1	0	3	0	3	0	3
545		7	max	0	1	.308	3	.107	3	0	1	0	3	0	1
546			min	-59.502	3	0	1	0	1	0	3	0	1	-.001	3
547		8	max	0	1	.205	3	.107	3	0	1	0	3	0	1
548			min	-59.556	3	0	1	0	1	0	3	0	1	-.001	3
549		9	max	0	1	.103	3	.107	3	0	1	0	3	0	1
550			min	-59.61	3	0	1	0	1	0	3	0	1	-.001	3
551		10	max	0	1	0	1	.107	3	0	1	0	3	0	1
552			min	-59.664	3	0	1	0	1	0	3	0	1	-.001	3
553		11	max	0	1	0	1	.107	3	0	1	0	3	0	1
554			min	-59.718	3	-.103	3	0	1	0	3	0	1	-.001	3
555		12	max	0	1	0	1	.107	3	0	1	0	3	0	1
556			min	-59.772	3	-.205	3	0	1	0	3	0	1	-.001	3
557		13	max	0	1	0	1	.107	3	0	1	0	3	0	1
558			min	-59.826	3	-.308	3	0	1	0	3	0	1	-.001	3
559		14	max	0	1	0	1	.107	3	0	1	0	3	0	1
560			min	-59.88	3	-.41	3	0	1	0	3	0	1	0	3
561		15	max	0	1	0	1	.107	3	0	1	0	3	0	1
562			min	-59.934	3	-.513	3	0	1	0	3	0	1	0	3
563		16	max	0	1	0	1	.107	3	0	1	0	3	0	1
564			min	-59.988	3	-.616	3	0	1	0	3	0	1	0	3
565		17	max	0	1	0	1	.107	3	0	1	0	3	0	1
566			min	-60.042	3	-.718	3	0	1	0	3	0	1	0	3
567		18	max	0	1	0	1	.107	3	0	1	0	3	0	1
568			min	-60.095	3	-.821	3	0	1	0	3	0	1	0	3
569		19	max	0	1	0	1	.107	3	0	1	0	3	0	1
570			min	-60.149	3	-.923	3	0	1	0	3	0	1	0	1
571	M16A	1	max	0	2	2.134	4	.233	4	0	3	0	3	0	1
572			min	-168.798	4	0	2	-.041	3	0	1	0	4	0	1
573		2	max	0	2	1.897	4	.211	4	0	3	0	3	0	2
574			min	-168.823	4	0	2	-.041	3	0	1	0	4	0	4
575		3	max	0	2	1.66	4	.19	4	0	3	0	3	0	2
576			min	-168.848	4	0	2	-.041	3	0	1	0	4	-.001	4
577		4	max	0	2	1.423	4	.168	4	0	3	0	3	0	2
578			min	-168.872	4	0	2	-.041	3	0	1	0	1	-.002	4
579		5	max	0	2	1.185	4	.147	4	0	3	0	3	0	2
580			min	-168.897	4	0	2	-.041	3	0	1	0	1	-.002	4
581		6	max	0	2	.948	4	.125	4	0	3	0	3	0	2
582			min	-168.922	4	0	2	-.041	3	0	1	0	1	-.002	4
583		7	max	0	2	.711	4	.104	4	0	3	0	3	0	2
584			min	-168.946	4	0	2	-.041	3	0	1	0	1	-.002	4
585		8	max	0	2	.474	4	.082	4	0	3	0	5	0	2
586			min	-168.971	4	0	2	-.041	3	0	1	0	1	-.003	4
587		9	max	0	2	.237	4	.06	4	0	3	0	5	0	2
588			min	-168.996	4	0	2	-.041	3	0	1	0	1	-.003	4
589		10	max	0	2	0	1	.04	1	0	3	0	5	0	2
590			min	-169.02	4	0	1	-.041	3	0	1	0	1	-.003	4
591		11	max	0	2	0	2	.04	1	0	3	0	5	0	2
592			min	-169.045	4	-.237	4	-.041	3	0	1	0	1	-.003	4
593		12	max	0	2	0	2	.04	1	0	3	0	5	0	2
594			min	-169.07	4	-.474	4	-.041	3	0	1	0	1	-.003	4
595		13	max	0	11	0	2	.04	1	0	3	0	5	0	2
596			min	-169.094	4	-.711	4	-.041	3	0	1	0	3	-.002	4
597		14	max	.06	11	0	2	.04	1	0	3	0	5	0	2
598			min	-169.119	4	-.948	4	-.052	5	0	1	0	3	-.002	4





RISA-3D Version 13.0.0      \...\...\PVMMini 60 Cell 1V 15° 120mph 30psf 4.5ft 7-05.rdb      Page 32



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	9	0	2	.007	5	4.81e-5	1	NC	1	NC	1
44			min	0	2	-.001	3	0	1	-6.221e-4	5	NC	1	NC	1
45		4	max	0	9	0	2	.01	4	5.756e-5	1	NC	1	NC	1
46			min	0	2	-.002	3	0	1	-6.262e-4	5	NC	1	NC	1
47		5	max	0	9	0	2	.013	4	6.701e-5	1	NC	1	NC	1
48			min	0	2	-.003	3	0	1	-6.303e-4	5	NC	1	NC	1
49		6	max	0	9	0	2	.016	4	7.647e-5	1	NC	1	NC	1
50			min	0	2	-.003	3	0	1	-6.345e-4	5	NC	1	NC	1
51		7	max	0	9	0	2	.02	4	8.593e-5	1	NC	1	NC	1
52			min	0	2	-.004	3	0	9	-6.386e-4	5	NC	1	NC	1
53		8	max	0	9	0	2	.023	4	9.539e-5	1	NC	1	NC	1
54			min	0	2	-.004	3	0	9	-6.427e-4	5	NC	1	NC	1
55		9	max	0	9	0	2	.026	4	1.048e-4	1	NC	1	NC	1
56			min	0	2	-.005	3	0	10	-6.469e-4	5	NC	1	NC	1
57		10	max	0	9	.001	2	.029	4	1.143e-4	1	NC	1	NC	1
58			min	0	2	-.005	3	0	10	-6.51e-4	5	NC	1	NC	1
59		11	max	0	9	.002	2	.032	4	1.238e-4	1	NC	1	NC	1
60			min	0	2	-.006	3	0	10	-6.551e-4	5	NC	1	NC	1
61		12	max	0	9	.002	2	.035	4	1.332e-4	1	NC	1	NC	1
62			min	0	2	-.006	3	0	10	-6.593e-4	5	NC	1	NC	1
63		13	max	0	9	.003	2	.038	4	1.427e-4	1	NC	1	NC	1
64			min	0	2	-.006	3	0	10	-6.634e-4	5	NC	1	NC	1
65		14	max	0	9	.004	2	.04	4	1.521e-4	1	NC	1	NC	1
66			min	0	2	-.006	3	0	10	-6.675e-4	5	NC	1	NC	1
67		15	max	0	9	.004	2	.043	4	1.616e-4	1	NC	1	NC	1
68			min	0	2	-.007	3	0	10	-6.717e-4	5	NC	1	NC	1
69		16	max	0	9	.005	2	.046	4	1.71e-4	1	NC	1	NC	1
70			min	0	2	-.007	3	0	10	-6.758e-4	5	8975.15	2	NC	1
71		17	max	0	9	.006	2	.048	4	1.805e-4	1	NC	3	NC	1
72			min	0	2	-.007	3	0	10	-6.799e-4	5	7639.53	2	NC	1
73		18	max	0	9	.007	2	.051	4	1.9e-4	1	NC	3	NC	1
74			min	0	2	-.007	3	0	10	-6.841e-4	5	6611.666	2	NC	1
75		19	max	0	9	.008	2	.053	4	1.994e-4	1	NC	3	NC	1
76			min	0	2	-.007	3	0	10	-6.882e-4	5	5811.84	2	NC	1
77	M4	1	max	.002	1	.006	2	0	10	2.28e-3	5	NC	1	NC	1
78			min	0	3	-.005	3	-.056	4	-2.145e-4	1	NC	1	343.765	4
79		2	max	.002	1	.006	2	0	10	2.28e-3	5	NC	1	NC	1
80			min	0	3	-.005	3	-.052	4	-2.145e-4	1	NC	1	374.719	4
81		3	max	.002	1	.006	2	0	10	2.28e-3	5	NC	1	NC	1
82			min	0	3	-.004	3	-.047	4	-2.145e-4	1	NC	1	411.557	4
83		4	max	.001	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
84			min	0	3	-.004	3	-.042	4	-2.145e-4	1	NC	1	455.828	4
85		5	max	.001	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
86			min	0	3	-.004	3	-.038	4	-2.145e-4	1	NC	1	509.643	4
87		6	max	.001	1	.005	2	0	10	2.28e-3	5	NC	1	NC	1
88			min	0	3	-.004	3	-.034	4	-2.145e-4	1	NC	1	575.938	4
89		7	max	.001	1	.004	2	0	10	2.28e-3	5	NC	1	NC	1
90			min	0	3	-.003	3	-.029	4	-2.145e-4	1	NC	1	658.893	4
91		8	max	.001	1	.004	2	0	10	2.28e-3	5	NC	1	NC	1
92			min	0	3	-.003	3	-.025	4	-2.145e-4	1	NC	1	764.629	4
93		9	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
94			min	0	3	-.003	3	-.021	4	-2.145e-4	1	NC	1	902.404	4
95		10	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
96			min	0	3	-.002	3	-.018	4	-2.145e-4	1	NC	1	1086.769	4
97		11	max	0	1	.003	2	0	10	2.28e-3	5	NC	1	NC	1
98			min	0	3	-.002	3	-.014	4	-2.145e-4	1	NC	1	1341.691	4
99		12	max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
100		min	0	3	-.002	3	-.011	4	-2.145e-4	1	NC	1	1708.972	4
101		max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1
102		min	0	3	-.002	3	-.009	4	-2.145e-4	1	NC	1	2267.018	4
103		max	0	1	.002	2	0	10	2.28e-3	5	NC	1	NC	1
104		min	0	3	-.001	3	-.006	4	-2.145e-4	1	NC	1	3177.55	4
105		max	0	1	.001	2	0	10	2.28e-3	5	NC	1	NC	1
106		min	0	3	-.001	3	-.004	4	-2.145e-4	1	NC	1	4820.49	4
107		max	0	1	.001	2	0	10	2.28e-3	5	NC	1	NC	1
108		min	0	3	0	3	-.002	4	-2.145e-4	1	NC	1	8275.825	4
109		max	0	1	0	2	0	10	2.28e-3	5	NC	1	NC	1
110		min	0	3	0	3	-.001	4	-2.145e-4	1	NC	1	NC	1
111		max	0	1	0	2	0	10	2.28e-3	5	NC	1	NC	1
112		min	0	3	0	3	0	4	-2.145e-4	1	NC	1	NC	1
113		max	0	1	0	1	0	1	2.28e-3	5	NC	1	NC	1
114		min	0	1	0	1	0	1	-2.145e-4	1	NC	1	NC	1
115	M6	max	.006	1	.018	2	0	1	1.064e-3	4	NC	3	NC	1
116		min	-.009	3	-.014	3	-.009	5	-8.516e-8	2	1651.802	2	8123.232	3
117		max	.006	1	.017	2	0	1	1.083e-3	4	NC	3	NC	1
118		min	-.008	3	-.013	3	-.009	5	-8.065e-8	2	1763.365	2	8703.294	3
119		max	.006	1	.016	2	0	1	1.102e-3	4	NC	3	NC	1
120		min	-.008	3	-.012	3	-.009	5	-7.614e-8	2	1890.696	2	9382.546	3
121		max	.005	1	.015	2	0	1	1.121e-3	4	NC	3	NC	1
122		min	-.007	3	-.012	3	-.008	5	-9.523e-7	11	2036.956	2	NC	1
123		max	.005	1	.014	2	0	1	1.14e-3	4	NC	3	NC	1
124		min	-.007	3	-.011	3	-.008	5	-1.978e-6	11	2206.217	2	NC	1
125		max	.005	1	.013	2	0	1	1.159e-3	4	NC	3	NC	1
126		min	-.006	3	-.01	3	-.008	5	-3.12e-6	1	2403.809	2	NC	1
127		max	.004	1	.011	2	0	1	1.178e-3	4	NC	3	NC	1
128		min	-.006	3	-.01	3	-.007	5	-5.463e-6	1	2636.846	2	NC	1
129		max	.004	1	.01	2	0	1	1.197e-3	4	NC	3	NC	1
130		min	-.005	3	-.009	3	-.007	5	-7.806e-6	1	2915.035	2	NC	1
131		max	.004	1	.009	2	0	1	1.215e-3	4	NC	3	NC	1
132		min	-.005	3	-.008	3	-.006	5	-1.015e-5	1	3251.973	2	NC	1
133		max	.003	1	.008	2	0	1	1.234e-3	4	NC	3	NC	1
134		min	-.004	3	-.007	3	-.006	5	-1.249e-5	1	3667.305	2	NC	1
135		max	.003	1	.007	2	0	1	1.253e-3	4	NC	3	NC	1
136		min	-.004	3	-.007	3	-.005	5	-1.483e-5	1	4190.505	2	NC	1
137		max	.003	1	.006	2	0	1	1.272e-3	4	NC	3	NC	1
138		min	-.003	3	-.006	3	-.005	5	-1.718e-5	1	4867.898	2	NC	1
139		max	.002	1	.005	2	0	1	1.291e-3	4	NC	3	NC	1
140		min	-.003	3	-.005	3	-.004	5	-1.952e-5	1	5776.694	2	NC	1
141		max	.002	1	.004	2	0	1	1.31e-3	4	NC	3	NC	1
142		min	-.002	3	-.004	3	-.003	5	-2.186e-5	1	7055.874	2	NC	1
143		max	.001	1	.003	2	0	1	1.329e-3	4	NC	1	NC	1
144		min	-.002	3	-.004	3	-.003	5	-2.421e-5	1	8983.406	2	NC	1
145		max	.001	1	.002	2	0	1	1.348e-3	4	NC	1	NC	1
146		min	-.001	3	-.003	3	-.002	5	-2.655e-5	1	NC	1	NC	1
147		max	0	1	.002	2	0	1	1.367e-3	4	NC	1	NC	1
148		min	0	3	-.002	3	-.001	5	-2.889e-5	1	NC	1	NC	1
149		max	0	1	0	2	0	1	1.386e-3	4	NC	1	NC	1
150		min	0	3	0	3	0	5	-3.123e-5	1	NC	1	NC	1
151		max	0	1	0	1	0	1	1.404e-3	4	NC	1	NC	1
152		min	0	1	0	1	0	1	-3.358e-5	1	NC	1	NC	1
153	M7	max	0	1	0	1	0	1	1.521e-5	1	NC	1	NC	1
154		min	0	1	0	1	0	1	-6.393e-4	4	NC	1	NC	1
155		max	0	9	.001	2	.003	4	1.391e-5	1	NC	1	NC	1
156		min	0	2	-.001	3	0	1	-6.303e-4	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157	3	max	0	9	.002	2	.007	4	1.262e-5	1	NC	1	NC	1
158		min	0	2	-.003	3	0	1	-6.212e-4	4	NC	1	NC	1
159	4	max	0	9	.003	2	.01	4	1.133e-5	1	NC	1	NC	1
160		min	0	2	-.004	3	0	1	-6.122e-4	4	NC	1	NC	1
161	5	max	0	9	.004	2	.014	4	1.004e-5	1	NC	1	NC	1
162		min	0	2	-.006	3	0	1	-6.032e-4	4	NC	1	NC	1
163	6	max	0	9	.006	2	.017	4	8.742e-6	1	NC	1	NC	1
164		min	0	2	-.007	3	0	1	-5.942e-4	4	8323.139	2	NC	1
165	7	max	0	9	.007	2	.021	4	2.339e-5	3	NC	3	NC	1
166		min	0	2	-.009	3	0	1	-5.852e-4	4	6887.693	2	NC	1
167	8	max	0	9	.008	2	.024	4	3.94e-5	3	NC	3	NC	1
168		min	0	2	-.01	3	0	1	-5.762e-4	4	5832.223	2	NC	1
169	9	max	0	9	.009	2	.027	4	5.541e-5	3	NC	3	NC	1
170		min	0	2	-.011	3	0	1	-5.672e-4	4	5019.427	2	NC	1
171	10	max	0	9	.011	2	.03	4	7.143e-5	3	NC	3	NC	1
172		min	-.001	2	-.012	3	0	1	-5.582e-4	4	4373.107	2	NC	1
173	11	max	0	9	.012	2	.033	4	8.744e-5	3	NC	3	NC	1
174		min	-.001	2	-.013	3	0	1	-5.492e-4	4	3847.352	2	NC	1
175	12	max	0	9	.013	2	.036	4	1.035e-4	3	NC	3	NC	1
176		min	-.001	2	-.015	3	0	1	-5.402e-4	4	3412.604	2	NC	1
177	13	max	0	9	.015	2	.039	4	1.195e-4	3	NC	3	NC	1
178		min	-.001	2	-.016	3	0	1	-5.312e-4	4	3048.764	2	NC	1
179	14	max	0	9	.017	2	.042	4	1.355e-4	3	NC	3	NC	1
180		min	-.002	2	-.016	3	0	1	-5.222e-4	4	2741.534	2	NC	1
181	15	max	0	9	.019	2	.045	4	1.515e-4	3	NC	3	NC	1
182		min	-.002	2	-.017	3	0	1	-5.132e-4	4	2480.356	2	NC	1
183	16	max	0	9	.02	2	.047	4	1.675e-4	3	NC	3	NC	1
184		min	-.002	2	-.018	3	0	1	-5.042e-4	4	2257.2	2	NC	1
185	17	max	.001	9	.022	2	.05	4	1.835e-4	3	NC	3	NC	1
186		min	-.002	2	-.019	3	0	1	-4.952e-4	4	2065.818	2	NC	1
187	18	max	.001	9	.024	2	.052	4	1.995e-4	3	NC	3	NC	1
188		min	-.002	2	-.02	3	0	1	-4.862e-4	4	1901.267	2	NC	1
189	19	max	.001	9	.026	2	.055	4	2.155e-4	3	NC	3	NC	1
190		min	-.002	2	-.02	3	0	1	-4.771e-4	4	1759.582	2	NC	1
191	M8	1	max	.005	1	.021	2	0	2.095e-3	4	NC	1	NC	1
192		min	-.002	3	-.015	3	-.057	4	-1.715e-4	3	NC	1	336.691	4
193	2	max	.005	1	.02	2	0	1	2.095e-3	4	NC	1	NC	1
194		min	-.002	3	-.014	3	-.053	4	-1.715e-4	3	NC	1	367.008	4
195	3	max	.005	1	.019	2	0	1	2.095e-3	4	NC	1	NC	1
196		min	-.002	3	-.013	3	-.048	4	-1.715e-4	3	NC	1	403.089	4
197	4	max	.005	1	.017	2	0	1	2.095e-3	4	NC	1	NC	1
198		min	-.002	3	-.013	3	-.043	4	-1.715e-4	3	NC	1	446.451	4
199	5	max	.004	1	.016	2	0	1	2.095e-3	4	NC	1	NC	1
200		min	-.002	3	-.012	3	-.039	4	-1.715e-4	3	NC	1	499.162	4
201	6	max	.004	1	.015	2	0	1	2.095e-3	4	NC	1	NC	1
202		min	-.001	3	-.011	3	-.034	4	-1.715e-4	3	NC	1	564.096	4
203	7	max	.004	1	.014	2	0	1	2.095e-3	4	NC	1	NC	1
204		min	-.001	3	-.01	3	-.03	4	-1.715e-4	3	NC	1	645.349	4
205	8	max	.003	1	.013	2	0	1	2.095e-3	4	NC	1	NC	1
206		min	-.001	3	-.009	3	-.026	4	-1.715e-4	3	NC	1	748.916	4
207	9	max	.003	1	.012	2	0	1	2.095e-3	4	NC	1	NC	1
208		min	-.001	3	-.008	3	-.022	4	-1.715e-4	3	NC	1	883.864	4
209	10	max	.003	1	.01	2	0	1	2.095e-3	4	NC	1	NC	1
210		min	0	3	-.008	3	-.018	4	-1.715e-4	3	NC	1	1064.449	4
211	11	max	.002	1	.009	2	0	1	2.095e-3	4	NC	1	NC	1
212		min	0	3	-.007	3	-.015	4	-1.715e-4	3	NC	1	1314.145	4
213	12	max	.002	1	.008	2	0	1	2.095e-3	4	NC	1	NC	1



RISA-3D Version 13.0.0    \...\PVMMini 60 Cell 1V 15° 120mph 30psf 4.5ft 7-05.rdb    Page 36





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	9	0	2	.005	4	4.864e-5	3	NC	1	NC	1
272			min	0	2	-.001	3	0	3	-5.668e-4	4	NC	1	9037.956	4
273		4	max	0	9	0	2	.008	4	3.272e-5	3	NC	1	NC	1
274			min	0	2	-.002	3	-.001	3	-6.166e-4	4	NC	1	5957.257	4
275		5	max	0	9	0	2	.01	4	1.68e-5	3	NC	1	NC	1
276			min	0	2	-.003	3	-.001	3	-6.664e-4	4	NC	1	4428.822	4
277		6	max	0	9	0	2	.013	4	8.768e-7	3	NC	1	NC	1
278			min	0	2	-.003	3	-.002	3	-7.163e-4	4	NC	1	3519.844	4
279		7	max	0	9	0	2	.016	4	-8.533e-7	10	NC	1	NC	1
280			min	0	2	-.004	3	-.002	3	-7.661e-4	4	NC	1	2919.538	4
281		8	max	0	9	0	2	.018	5	-9.299e-7	10	NC	1	NC	1
282			min	0	2	-.005	3	-.002	3	-8.159e-4	4	NC	1	2494.783	5
283		9	max	0	9	0	2	.021	5	-1.007e-6	10	NC	1	NC	1
284			min	0	2	-.005	3	-.002	3	-8.657e-4	4	NC	1	2176.64	5
285		10	max	0	9	.001	2	.024	5	-1.083e-6	10	NC	1	NC	1
286			min	0	2	-.006	3	-.002	3	-9.156e-4	4	NC	1	1931.436	5
287		11	max	0	9	.002	2	.026	5	-1.16e-6	10	NC	1	NC	1
288			min	0	2	-.006	3	-.002	3	-9.654e-4	4	NC	1	1736.797	5
289		12	max	0	9	.002	2	.029	5	-1.236e-6	10	NC	1	NC	1
290			min	0	2	-.006	3	-.002	3	-1.015e-3	4	NC	1	1578.494	5
291		13	max	0	9	.003	2	.032	5	-1.313e-6	10	NC	1	NC	1
292			min	0	2	-.006	3	-.002	3	-1.065e-3	4	NC	1	1447.049	5
293		14	max	0	9	.004	2	.034	5	-1.389e-6	10	NC	1	NC	1
294			min	0	2	-.007	3	-.002	3	-1.115e-3	4	NC	1	1335.899	5
295		15	max	0	9	.004	2	.037	5	-1.466e-6	10	NC	1	NC	1
296			min	0	2	-.007	3	-.002	3	-1.165e-3	4	NC	1	1240.359	5
297		16	max	0	9	.005	2	.04	5	-1.543e-6	10	NC	1	NC	1
298			min	0	2	-.007	3	-.002	1	-1.215e-3	4	8987.357	2	1156.991	5
299		17	max	0	9	.006	2	.042	5	-1.619e-6	10	NC	3	NC	1
300			min	0	2	-.007	3	-.003	1	-1.264e-3	4	7648.752	2	1083.222	5
301		18	max	0	9	.007	2	.045	5	-1.696e-6	10	NC	3	NC	1
302			min	0	2	-.007	3	-.003	1	-1.314e-3	4	6618.851	2	1017.086	5
303		19	max	0	9	.008	2	.048	5	-1.772e-6	10	NC	3	NC	1
304			min	0	2	-.007	3	-.003	1	-1.364e-3	4	5817.606	2	957.069	5
305	M12	1	max	.002	1	.006	2	.003	1	2.846e-3	4	NC	1	NC	2
306			min	0	3	-.005	3	-.053	5	1.509e-6	10	NC	1	366.538	5
307		2	max	.002	1	.006	2	.002	1	2.846e-3	4	NC	1	NC	2
308			min	0	3	-.005	3	-.048	5	1.509e-6	10	NC	1	399.534	5
309		3	max	.002	1	.006	2	.002	1	2.846e-3	4	NC	1	NC	2
310			min	0	3	-.004	3	-.044	5	1.509e-6	10	NC	1	438.802	5
311		4	max	.001	1	.005	2	.002	1	2.846e-3	4	NC	1	NC	1
312			min	0	3	-.004	3	-.04	5	1.509e-6	10	NC	1	485.992	5
313		5	max	.001	1	.005	2	.002	1	2.846e-3	4	NC	1	NC	1
314			min	0	3	-.004	3	-.036	5	1.509e-6	10	NC	1	543.355	5
315		6	max	.001	1	.005	2	.001	1	2.846e-3	4	NC	1	NC	1
316			min	0	3	-.004	3	-.031	5	1.509e-6	10	NC	1	614.019	5
317		7	max	.001	1	.004	2	.001	1	2.846e-3	4	NC	1	NC	1
318			min	0	3	-.003	3	-.028	5	1.509e-6	10	NC	1	702.44	5
319		8	max	.001	1	.004	2	.001	1	2.846e-3	4	NC	1	NC	1
320			min	0	3	-.003	3	-.024	5	1.509e-6	10	NC	1	815.141	5
321		9	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
322			min	0	3	-.003	3	-.02	5	1.509e-6	10	NC	1	961.989	5
323		10	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
324			min	0	3	-.002	3	-.017	5	1.509e-6	10	NC	1	1158.492	5
325		11	max	0	1	.003	2	0	1	2.846e-3	4	NC	1	NC	1
326			min	0	3	-.002	3	-.014	5	1.509e-6	10	NC	1	1430.193	5
327		12	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328			min	0	3	-.002	3	-.011	5	1.509e-6	10	NC	1	1821.641	5
329		13	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1
330			min	0	3	-.002	3	-.008	5	1.509e-6	10	NC	1	2416.397	5
331		14	max	0	1	.002	2	0	1	2.846e-3	4	NC	1	NC	1
332			min	0	3	-.001	3	-.006	5	1.509e-6	10	NC	1	3386.81	5
333		15	max	0	1	.001	2	0	1	2.846e-3	4	NC	1	NC	1
334			min	0	3	-.001	3	-.004	5	1.509e-6	10	NC	1	5137.769	5
335		16	max	0	1	.001	2	0	1	2.846e-3	4	NC	1	NC	1
336			min	0	3	0	3	-.002	5	1.509e-6	10	NC	1	8820.212	5
337		17	max	0	1	0	2	0	1	2.846e-3	4	NC	1	NC	1
338			min	0	3	0	3	-.001	5	1.509e-6	10	NC	1	NC	1
339		18	max	0	1	0	2	0	1	2.846e-3	4	NC	1	NC	1
340			min	0	3	0	3	0	5	1.509e-6	10	NC	1	NC	1
341		19	max	0	1	0	1	0	1	2.846e-3	4	NC	1	NC	1
342			min	0	1	0	1	0	1	1.509e-6	10	NC	1	NC	1
343	M1	1	max	.005	3	.021	3	.005	5	8.376e-3	1	NC	1	NC	1
344			min	-.005	2	-.019	1	0	1	-1.034e-2	3	NC	1	NC	1
345		2	max	.005	3	.011	3	.007	5	4.085e-3	1	NC	4	NC	1
346			min	-.005	2	-.01	1	-.002	1	-5.09e-3	3	4999.456	3	NC	1
347		3	max	.005	3	.002	3	.01	5	1.943e-4	5	NC	4	NC	1
348			min	-.005	2	-.002	1	-.003	1	-1.259e-4	1	2596.174	3	9993.366	5
349		4	max	.005	3	.006	2	.012	5	1.873e-4	5	NC	4	NC	1
350			min	-.006	2	-.005	3	-.003	1	-1.017e-4	1	1857.912	2	6294.748	5
351		5	max	.005	3	.012	2	.015	5	1.803e-4	5	NC	4	NC	1
352			min	-.006	2	-.011	3	-.003	1	-7.741e-5	1	1476.081	2	4497.945	5
353		6	max	.005	3	.017	2	.018	5	1.732e-4	5	NC	4	NC	1
354			min	-.006	2	-.016	3	-.003	1	-5.316e-5	1	1258.491	2	3452.202	5
355		7	max	.005	3	.021	2	.022	5	1.662e-4	5	NC	5	NC	1
356			min	-.006	2	-.02	3	-.003	1	-2.916e-5	9	1124.869	2	2776.777	5
357		8	max	.005	3	.024	2	.025	5	1.592e-4	5	NC	5	NC	1
358			min	-.006	2	-.022	3	-.002	1	-1.195e-5	9	1041.714	2	2309.717	5
359		9	max	.005	3	.026	2	.029	5	1.551e-4	4	NC	5	NC	1
360			min	-.006	2	-.023	3	-.002	1	7.908e-7	10	993.246	2	1965.934	4
361		10	max	.005	3	.027	2	.032	4	1.534e-4	4	NC	5	NC	1
362			min	-.006	2	-.024	3	0	1	9.576e-7	10	971.99	2	1700.479	4
363		11	max	.005	3	.027	2	.036	4	1.517e-4	4	NC	5	NC	1
364			min	-.006	2	-.023	3	0	9	1.124e-6	10	975.335	2	1497.669	4
365		12	max	.005	3	.025	2	.04	4	1.5e-4	4	NC	5	NC	1
366			min	-.006	2	-.021	3	0	10	1.291e-6	10	1004.535	2	1339.344	4
367		13	max	.005	3	.022	2	.043	4	1.484e-4	4	NC	5	NC	1
368			min	-.006	2	-.018	3	0	10	1.458e-6	10	1065.34	2	1213.681	4
369		14	max	.005	3	.018	2	.047	4	1.467e-4	4	NC	4	NC	1
370			min	-.006	2	-.014	3	0	10	1.625e-6	10	1170.847	2	1112.691	4
371		15	max	.005	3	.012	2	.05	4	1.651e-4	1	NC	4	NC	1
372			min	-.006	2	-.01	3	0	10	1.792e-6	10	1349.589	2	1030.825	4
373		16	max	.005	3	.005	2	.053	4	3.188e-4	4	NC	4	NC	1
374			min	-.006	2	-.004	3	0	10	1.935e-6	10	1670.603	2	964.128	4
375		17	max	.005	3	.002	3	.056	4	4.666e-3	4	NC	4	NC	1
376			min	-.006	2	-.003	2	0	10	1.513e-6	10	2348.385	2	909.779	4
377		18	max	.005	3	.009	3	.059	4	5.271e-3	2	NC	4	NC	1
378			min	-.006	2	-.013	2	0	10	-2.595e-3	3	4536.925	2	865.535	4
379		19	max	.005	3	.016	3	.061	4	1.061e-2	2	NC	1	NC	1
380			min	-.006	2	-.024	2	0	1	-5.276e-3	3	NC	1	830.737	4
381	M5	1	max	.014	3	.067	3	.005	5	8.041e-6	4	NC	1	NC	1
382			min	-.018	2	-.063	1	0	1	0	1	NC	1	NC	1
383		2	max	.014	3	.036	3	.007	5	9.064e-5	5	NC	4	NC	1
384			min	-.018	2	-.033	1	0	1	-1.952e-5	1	1561.503	3	NC	1



Company : Schletter, Inc.  
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Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.014	3	.008	3	.009	5	1.719e-4	5	NC	5	NC	1
386			min	-.018	2	-.005	1	0	1	-3.865e-5	1	805.689	1	NC	1
387		4	max	.014	3	.019	2	.012	5	1.789e-4	5	NC	5	NC	1
388			min	-.018	2	-.016	3	0	1	-3.644e-5	9	568.137	1	NC	1
389		5	max	.014	3	.039	2	.016	5	1.859e-4	5	NC	5	NC	1
390			min	-.018	2	-.036	3	0	1	-3.425e-5	9	453.8	1	NC	1
391		6	max	.014	3	.056	2	.019	5	1.929e-4	5	NC	5	NC	1
392			min	-.018	2	-.051	3	0	1	-3.207e-5	9	387.21	2	NC	1
393		7	max	.014	3	.07	2	.023	5	1.999e-4	5	NC	5	NC	1
394			min	-.019	2	-.063	3	0	1	-2.988e-5	9	345.95	2	NC	1
395		8	max	.014	3	.08	2	.027	5	2.069e-4	5	NC	5	NC	1
396			min	-.019	2	-.07	3	0	1	-2.77e-5	9	320.26	2	NC	1
397		9	max	.014	3	.086	2	.03	4	2.138e-4	5	NC	5	NC	1
398			min	-.019	2	-.074	3	0	1	-2.551e-5	9	305.266	2	NC	1
399		10	max	.014	3	.089	2	.034	4	2.208e-4	5	NC	5	NC	1
400			min	-.019	2	-.075	3	0	1	-2.333e-5	9	298.661	2	NC	1
401		11	max	.014	3	.087	2	.038	4	2.278e-4	5	NC	5	NC	1
402			min	-.019	2	-.072	3	0	1	-2.114e-5	9	299.635	2	NC	1
403		12	max	.014	3	.082	2	.042	4	2.348e-4	5	NC	5	NC	1
404			min	-.019	2	-.066	3	0	1	-1.896e-5	9	308.571	2	NC	1
405		13	max	.014	3	.072	2	.045	4	2.418e-4	5	NC	5	NC	1
406			min	-.019	2	-.057	3	0	1	-1.677e-5	9	327.234	2	NC	1
407		14	max	.014	3	.058	2	.049	4	2.488e-4	5	NC	5	NC	1
408			min	-.019	2	-.046	3	0	1	-1.459e-5	9	359.653	2	NC	1
409		15	max	.014	3	.04	2	.052	4	2.562e-4	4	NC	5	NC	1
410			min	-.019	2	-.031	3	0	1	-1.24e-5	9	414.612	2	NC	1
411		16	max	.014	3	.017	2	.055	4	4.364e-4	4	NC	5	NC	1
412			min	-.019	2	-.014	3	0	1	-1.124e-5	9	513.377	2	NC	1
413		17	max	.014	3	.005	3	.057	4	4.714e-3	4	NC	5	NC	1
414			min	-.019	2	-.01	2	0	1	-3.807e-5	1	722.199	2	NC	1
415		18	max	.014	3	.027	3	.059	4	2.421e-3	4	NC	4	NC	1
416			min	-.019	2	-.042	2	0	1	-1.957e-5	1	1395.832	2	NC	1
417		19	max	.014	3	.05	3	.061	4	3.48e-6	5	NC	1	NC	1
418			min	-.019	2	-.077	2	0	1	-3.386e-7	3	NC	1	NC	1
419	M9	1	max	.005	3	.021	3	.004	5	1.035e-2	3	NC	1	NC	1
420			min	-.005	2	-.019	1	-.001	1	-8.376e-3	1	NC	1	NC	1
421		2	max	.005	3	.011	3	.004	5	5.141e-3	3	NC	4	NC	1
422			min	-.005	2	-.01	1	0	9	-4.13e-3	1	5001.531	3	NC	1
423		3	max	.005	3	.002	3	.004	4	3.768e-5	1	NC	4	NC	1
424			min	-.005	2	-.002	1	0	3	-3.524e-5	5	2597.281	3	NC	1
425		4	max	.005	3	.006	2	.005	4	2.022e-5	3	NC	4	NC	1
426			min	-.005	2	-.005	3	-.001	3	-5.203e-5	5	1858.254	2	NC	1
427		5	max	.005	3	.012	2	.006	4	1.065e-5	2	NC	4	NC	1
428			min	-.005	2	-.011	3	-.002	3	-7.205e-5	4	1476.367	2	NC	1
429		6	max	.005	3	.017	2	.008	4	4.476e-6	2	NC	4	NC	1
430			min	-.006	2	-.016	3	-.002	3	-9.274e-5	4	1258.747	2	NC	1
431		7	max	.005	3	.021	2	.011	4	-3.197e-7	10	NC	5	NC	1
432			min	-.006	2	-.02	3	-.003	3	-1.134e-4	4	1125.108	2	6640.443	4
433		8	max	.005	3	.024	2	.014	4	-4.945e-7	10	NC	5	NC	1
434			min	-.006	2	-.022	3	-.003	3	-1.341e-4	4	1041.945	2	4610.767	4
435		9	max	.005	3	.026	2	.017	4	-6.693e-7	10	NC	5	NC	1
436			min	-.006	2	-.023	3	-.003	3	-1.548e-4	4	993.475	2	3428.712	4
437		10	max	.005	3	.027	2	.021	5	-8.44e-7	10	NC	5	NC	1
438			min	-.006	2	-.024	3	-.003	3	-1.755e-4	4	972.222	2	2676.895	4
439		11	max	.005	3	.027	2	.026	5	-1.019e-6	10	NC	5	NC	1
440			min	-.006	2	-.023	3	-.003	3	-1.962e-4	4	975.576	2	2167.787	4
441		12	max	.005	3	.025	2	.03	5	-1.194e-6	10	NC	5	NC	1









Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
556		min	-0.005	4	-0.013	1	-0.033	3	-5.265e-3	1	4926.327	2	1430.914	3
557	13	max	0	3	.003	5	.031	1	5.86e-3	3	NC	5	NC	15
558		min	-0.006	4	-0.012	1	-0.033	3	-5.742e-3	1	5334.953	2	1414.865	3
559	14	max	0	3	.003	5	.029	1	6.323e-3	3	NC	5	NC	5
560		min	-0.006	4	-0.011	1	-0.031	3	-6.22e-3	1	6015.827	2	1457.321	3
561	15	max	0	3	.003	5	.025	1	6.785e-3	3	NC	5	NC	5
562		min	-0.007	4	-0.01	1	-0.027	3	-6.697e-3	1	7148.035	2	1580.676	3
563	16	max	0	3	.003	5	.019	1	7.247e-3	3	NC	3	NC	4
564		min	-0.007	4	-0.008	1	-.02	3	-7.175e-3	1	9160.505	2	1846.078	3
565	17	max	0	3	.004	5	.01	1	7.709e-3	3	NC	1	NC	4
566		min	-0.008	4	-0.006	1	-0.011	3	-7.652e-3	1	NC	1	2445.627	3
567	18	max	0	3	.004	5	.001	9	8.172e-3	3	NC	1	NC	4
568		min	-0.008	4	-0.004	9	-0.005	2	-8.13e-3	1	NC	1	4351.405	3
569	19	max	0	3	.004	3	.016	3	8.634e-3	3	NC	1	NC	1
570		min	-0.009	4	-0.002	9	-0.019	2	-8.607e-3	1	NC	1	NC	1
571	M16A	1	max	0	10	0	.005	3	2.558e-3	3	NC	1	NC	1
572		min	-0.003	4	-0.002	4	-0.006	2	-2.563e-3	2	NC	1	NC	1
573	2	max	0	10	0	12	.001	9	2.447e-3	3	NC	1	NC	1
574		min	-0.003	4	-0.006	4	-0.001	5	-2.443e-3	2	NC	1	NC	1
575	3	max	0	10	-0.002	12	.005	1	2.336e-3	3	NC	1	NC	4
576		min	-0.002	4	-.01	4	-0.004	3	-2.323e-3	2	7703.344	4	6935.617	3
577	4	max	0	10	-0.003	12	.008	1	2.225e-3	3	NC	3	NC	4
578		min	-0.002	4	-0.014	4	-0.008	3	-2.203e-3	2	5284.944	4	5266.081	3
579	5	max	0	10	-0.004	12	.009	1	2.113e-3	3	NC	12	NC	9
580		min	-0.002	4	-0.017	4	-.01	5	-2.083e-3	2	4123.895	4	4539.148	3
581	6	max	0	10	-0.005	12	.011	1	2.002e-3	3	NC	12	NC	10
582		min	-0.002	4	-.02	4	-0.014	5	-1.963e-3	2	3470.694	4	4217.064	3
583	7	max	0	10	-0.006	12	.011	1	1.891e-3	3	NC	12	NC	10
584		min	-0.002	4	-0.022	4	-0.017	5	-1.843e-3	2	3077.879	4	3743.361	5
585	8	max	0	10	-0.006	12	.011	1	1.78e-3	3	9852.653	12	NC	10
586		min	-0.002	4	-0.023	4	-.02	5	-1.723e-3	2	2842.131	4	3140.171	5
587	9	max	0	10	-0.006	12	.01	1	1.669e-3	3	9412.759	12	NC	9
588		min	-0.002	4	-0.024	4	-0.023	5	-1.603e-3	2	2715.238	4	2786.033	5
589	10	max	0	10	-0.006	12	.009	1	1.558e-3	3	9273.598	12	NC	9
590		min	-0.001	4	-0.024	4	-0.024	5	-1.483e-3	2	2675.095	4	2592.011	5
591	11	max	0	10	-0.006	12	.008	1	1.447e-3	3	9412.759	12	NC	9
592		min	-0.001	4	-0.024	4	-0.025	5	-1.363e-3	2	2715.238	4	2517.781	5
593	12	max	0	10	-0.006	12	.007	1	1.335e-3	3	9852.653	12	NC	9
594		min	-0.001	4	-0.023	4	-0.025	5	-1.243e-3	2	2842.131	4	2550.569	5
595	13	max	0	10	-0.006	12	.005	1	1.224e-3	3	NC	12	NC	2
596		min	0	4	-0.021	4	-0.023	5	-1.123e-3	2	3077.879	4	2699.839	5
597	14	max	0	10	-0.005	12	.004	1	1.113e-3	3	NC	12	NC	1
598		min	0	4	-0.018	4	-0.021	5	-1.004e-3	2	3470.694	4	3003.473	5
599	15	max	0	10	-0.004	12	.002	1	1.002e-3	3	NC	12	NC	1
600		min	0	4	-0.016	4	-0.018	5	-8.835e-4	2	4123.895	4	3553.224	5
601	16	max	0	10	-0.003	12	.001	1	8.909e-4	3	NC	3	NC	1
602		min	0	4	-0.012	4	-0.014	5	-7.636e-4	2	5284.944	4	4577.779	5
603	17	max	0	10	-0.002	12	0	9	7.797e-4	3	NC	1	NC	1
604		min	0	4	-0.008	4	-0.009	5	-6.436e-4	2	7703.344	4	6780.912	5
605	18	max	0	10	-0.001	12	0	9	7.187e-4	4	NC	1	NC	1
606		min	0	4	-0.004	4	-0.005	5	-5.237e-4	2	NC	1	NC	1
607	19	max	0	1	0	1	0	1	7.789e-4	4	NC	1	NC	1
608		min	0	1	0	1	0	1	-4.037e-4	2	NC	1	NC	1



**Anchor Designer™**  
Software  
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™  
Software  
Version 2.4.5673.0

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Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 405  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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Address:			
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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657





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## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
<b>Adhesive</b>	<b>405</b>	<b>5365</b>	<b>0.08</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>101</b>	<b>3156</b>	<b>0.03</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.





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### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





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## 3. Resulting Anchor Forces

Anchor	Tension load, N <sub>ua</sub> (lb)	Shear load x, V <sub>uax</sub> (lb)	Shear load y, V <sub>uay</sub> (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1465  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis, e<sub>Nx</sub> (inch): 0.00  
Eccentricity of resultant tension forces in y-axis, e<sub>Ny</sub> (inch): 0.00  
Eccentricity of resultant shear forces in x-axis, e<sub>Vx</sub> (inch): 0.00  
Eccentricity of resultant shear forces in y-axis, e<sub>Vy</sub> (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N <sub>sa</sub> (lb)	φ	φN <sub>sa</sub> (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k <sub>c</sub>	λ	f <sub>c</sub> (psi)	h <sub>ef</sub> (in)	N <sub>b</sub> (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A <sub>Nc</sub> (in <sup>2</sup> )	A <sub>Nco</sub> (in <sup>2</sup> )	ψ <sub>ec,N</sub>	ψ <sub>ed,N</sub>	ψ <sub>c,N</sub>	ψ <sub>cp,N</sub>	N <sub>b</sub> (lb)	φ	φN <sub>cbg</sub> (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ <sub>k,cr</sub> (psi)	f <sub>short-term</sub>	K <sub>sat</sub>	τ <sub>k,cr</sub> (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ <sub>k,cr</sub> (psi)	d <sub>a</sub> (in)	h <sub>ef</sub> (in)	N <sub>a0</sub> (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A <sub>Na</sub> (in <sup>2</sup> )	A <sub>Na0</sub> (in <sup>2</sup> )	ψ <sub>ed,Na</sub>	ψ <sub>g,Na</sub>	ψ <sub>ec,Na</sub>	ψ <sub>p,Na</sub>	N <sub>a0</sub> (lb)	φ	φN <sub>ag</sub> (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{at}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

$\phi V_{cpg}$ (lb)
15580

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N <sub>ua</sub> (lb)	Design Strength, ϕN <sub>n</sub> (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V <sub>ua</sub> (lb)	Design Strength, ϕV <sub>n</sub> (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N <sub>ua</sub> /ϕN <sub>n</sub>	V <sub>ua</sub> /ϕV <sub>n</sub>	Combined Ratio	Permissible	Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

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