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## 1. INTRODUCTION

### 1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMax ground mount system.

### 1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 2  
Module Tilt = 20°  
Maximum Height Above Grade = 3 ft

### 1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

## 2. LOAD ACTIONS

### 2.1 Permanent Loads

$g_{MAX}$ =	3.00 psf
$g_{MIN}$ =	1.75 psf

Self-weight of the PV modules.

### 2.2 Snow Loads

Ground Snow Load, $P_g$ =	30.00 psf	
Sloped Roof Snow Load, $P_s$ =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
$I_s$ =	1.00	
$C_s$ =	0.91	
$C_e$ =	0.90	
$C_t$ =	1.20	

### 2.3 Wind Loads

Design Wind Speed, $V$ =	160 mph	Exposure Category = C
Height <	15 ft	Importance Category = II

Peak Velocity Pressure,  $q_z$  = 40.19 psf Including the gust factor,  $G=0.85$ . (ASCE 7-10, Eq. 27.3-1)

### Pressure Coefficients

$C_{f+ TOP}$ =	1.050	(Pressure)
$C_{f+ BOTTOM}$ =	1.650	
$C_{f- TOP, OUTER PURLIN}$ =	-2.400	
$C_{f- TOP, INNER PURLIN}$ =	-1.840	(Suction)
$C_{f- BOTTOM}$ =	-1.000	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

### 2.4 Seismic Loads - N/A

$S_S$ =	0.00	$R$ = 1.25
$S_{DS}$ =	0.00	$C_s$ = 0
$S_1$ =	0.00	$\rho$ = 1.3
$S_{D1}$ =	0.00	$\Omega$ = 1.25
$T_a$ =	0.00	$C_d$ = 1.25

ASCE 7, Section 12.8.1.3: A maximum  $S_S$  of 1.5 may be used to calculate the base shear,  $C_s$ , of structures under five stories and with a period,  $T$ , of 0.5 or less. Therefore, a  $S_{ds}$  of 1.0 was used to calculate  $C_s$ .



Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

## 2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

### Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

### Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& } (\text{ASCE 7, Section 12.4.3.2}) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

<sup>M</sup> Uses the minimum allowable module dead load.

<sup>R</sup> Include redundancy factor of 1.3.

<sup>O</sup> Includes overstrength factor of 1.25. Used to check seismic drift.

## 3. STRUCTURAL ANALYSIS

### 3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

### 3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M14	Mid-Top	M7	Inner	N15	Inner
M15	Mid-Bottom	M11	Outer	N23	Outer
M16	Bottom				
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>				
M4	Outer				
M8	Inner				
M12	Outer				

## 4. MEMBER DESIGN CALCULATIONS

### 4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

Purlin Type =	<b>S1.5</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	102 in
$\Phi F_{ty}$ STRONG-AXIS =	25.07 ksi
$\Phi F_{ty}$ WEAK-AXIS =	23.08 ksi
$S_y$ =	1.33 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.16 in <sup>4</sup>
$I_x$ =	1.07 in <sup>4</sup>
$A$ =	1.25 in <sup>2</sup>
$g$ =	1.50 lbs/ft
$M_y$ =	-2.379 k-ft
$M_z$ =	0.004 k-ft
$M_{y \text{ allowable}}$ =	2.779 k-ft
$M_{z \text{ allowable}}$ =	1.154 k-ft
Utilization =	<b>86%</b>

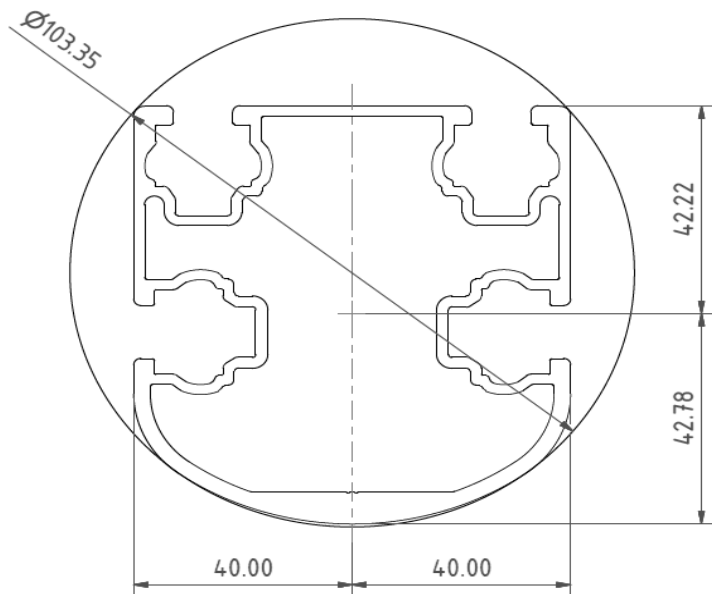


DETAIL VIEW

### 4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	<b>BF0</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	88.90 in
$\Phi F_{ty}$ AXIAL =	31.09 ksi
$\Phi F_{ty}$ STRONG-AXIS =	29.35 ksi
$\Phi F_{ty}$ WEAK-AXIS =	33.25 ksi
$S_y$ =	1.42 in <sup>3</sup>
$S_x$ =	1.41 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	2.39 in <sup>4</sup>
$I_x$ =	2.22 in <sup>4</sup>
$A$ =	1.88 in <sup>2</sup>
$g$ =	2.26 lbs/ft
$M_y$ =	-3.380 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	-0.739 k
$M_{y \text{ allowable}}$ =	3.464 k-ft
$M_{z \text{ allowable}}$ =	3.907 k-ft
$P_{n \text{ allowable}}$ =	58.535 k
Utilization =	<b>99%</b>



### 4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M12 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

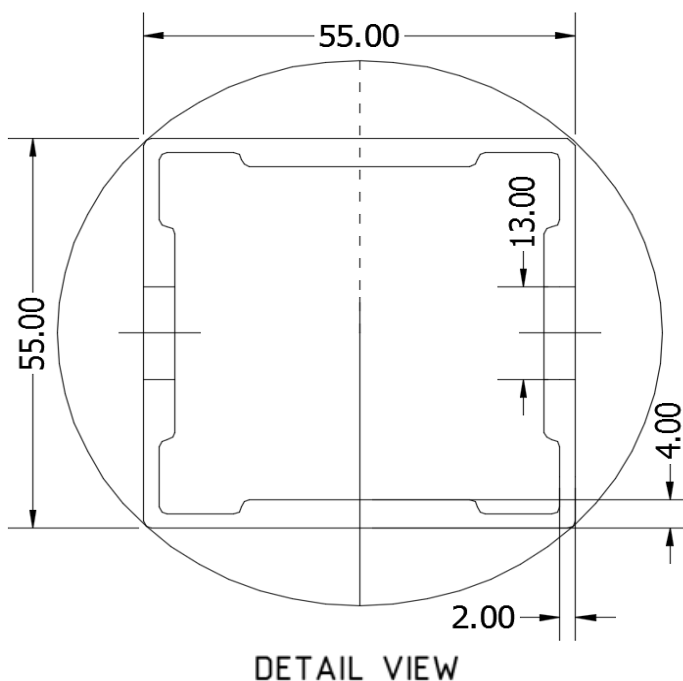
Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	24.80 in
$\Phi F_{ty \text{ AXIAL}}$ =	28.03 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.000 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.473 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	27.532 k
Utilization =	<b>13%</b>



### 4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M12 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	86.60 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.50 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	2.083 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	7.371 k
Utilization =	<b>29%</b>



#### 4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M12 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	<b>55x55</b>
Aluminum Type =	6105-T5
$F_{ty}$ =	35 ksi
$L_b$ =	55.91 in
$\Phi F_{ty \text{ AXIAL}}$ =	15.92 ksi
$\Phi F_{ty \text{ BENDING}}$ =	28.22 ksi
$S_y$ =	0.60 in <sup>3</sup>
$S_x$ =	0.60 in <sup>3</sup>
$E$ =	10100 ksi
$I_y$ =	0.67 in <sup>4</sup>
$I_x$ =	0.67 in <sup>4</sup>
$A$ =	0.98 in <sup>2</sup>
$g$ =	1.18 lbs/ft
$M_y$ =	-0.011 k-ft
$M_z$ =	0.000 k-ft
$P_n$ =	3.553 k
$M_{y \text{ allowable}}$ =	1.408 k-ft
$M_{z \text{ allowable}}$ =	1.408 k-ft
$P_{n \text{ allowable}}$ =	15.642 k
Utilization =	<u>23%</u>



### 5. FOUNDATION DESIGN CALCULATIONS

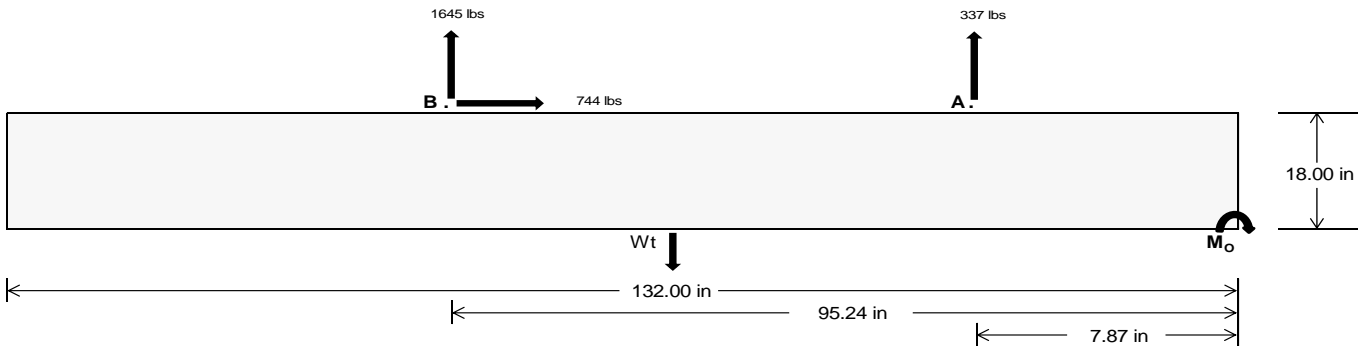
#### 5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>1478.66</u>	<u>7144.22</u>	k
Compressive Load =	<u>4515.45</u>	<u>5355.74</u>	k
Lateral Load =	<u>8.68</u>	<u>3224.97</u>	k
Moment (Weak Axis) =	<u>0.02</u>	<u>0.00</u>	k

## 5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



### Concrete Properties

Weight of Concrete = 145 pcf  
Compressive Strength = 2500 psi  
Yield Strength = 60000 psi

### Overturning Check

$M_o = 172687.8$  in-lbs  
Resisting Force Required = 2616.48 lbs  
S.F. = 1.67  
Weight Required = 4360.80 lbs  
Minimum Width = 38 in  
Weight Provided = 7576.25 lbs

### Sliding

Force = 744.05 lbs  
Friction = 0.4  
Weight Required = 1860.13 lbs  
Resisting Weight = 7576.25 lbs  
Additional Weight Required = 0 lbs

### Cohesion

Sliding Force = 744.05 lbs  
Cohesion = 130 psf  
Area = 34.83 ft<sup>2</sup>  
Resisting = 3788.13 lbs  
Additional Weight Required = 0 lbs

### Shear Key

Additional Force = 0 lbs  
Lateral Bearing Pressure = 200 psf/ft  
Required Depth = 0.00 ft  
 $f'_c = 2500$  psi  
Length = 8 in

### Footing Reinforcement

Use fiber reinforcing with (3) #5 rebar.

A minimum 132in long x 38in wide x 18in tall ballast foundation is required to resist overturning.

Use a 132in long x 38in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 132in long x 38in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

### Bearing Pressure

Ballast Width  
 $P_{ftg} = (145 \text{ pcf})(11 \text{ ft})(1.5 \text{ ft})(3.17 \text{ ft}) =$   
38 in 39 in 40 in 41 in  
7576 lbs 7776 lbs 7975 lbs 8174 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in	38 in	39 in	40 in	41 in
$F_A$	1320 lbs	1320 lbs	1320 lbs	1320 lbs	1816 lbs	1816 lbs	1816 lbs	1816 lbs	2242 lbs	2242 lbs	2242 lbs	2242 lbs	-674 lbs	-674 lbs	-674 lbs	-674 lbs
$F_B$	1340 lbs	1340 lbs	1340 lbs	1340 lbs	2199 lbs	2199 lbs	2199 lbs	2199 lbs	2543 lbs	2543 lbs	2543 lbs	2543 lbs	-3290 lbs	-3290 lbs	-3290 lbs	-3290 lbs
$F_V$	143 lbs	143 lbs	143 lbs	143 lbs	1316 lbs	1316 lbs	1316 lbs	1316 lbs	1084 lbs	1084 lbs	1084 lbs	1084 lbs	-1488 lbs	-1488 lbs	-1488 lbs	-1488 lbs
$P_{total}$	10236 lbs	10436 lbs	10636 lbs	10834 lbs	11591 lbs	11790 lbs	11990 lbs	12189 lbs	12361 lbs	12560 lbs	12759 lbs	12959 lbs	582 lbs	702 lbs	822 lbs	941 lbs
$M$	3342 lbs-ft	3342 lbs-ft	3342 lbs-ft	3342 lbs-ft	5416 lbs-ft	5416 lbs-ft	5416 lbs-ft	5416 lbs-ft	6289 lbs-ft	6289 lbs-ft	6289 lbs-ft	6289 lbs-ft	2518 lbs-ft	2518 lbs-ft	2518 lbs-ft	2518 lbs-ft
$e$	0.33 ft	0.32 ft	0.31 ft	0.31 ft	0.47 ft	0.46 ft	0.45 ft	0.44 ft	0.51 ft	0.50 ft	0.49 ft	0.49 ft	4.32 ft	3.59 ft	3.06 ft	2.68 ft
$L/6$	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
$f_{min}$	241.5 psf	240.9 psf	240.3 psf	239.8 psf	247.9 psf	247.2 psf	246.4 psf	245.7 psf	256.4 psf	255.4 psf	254.4 psf	253.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
$f_{max}$	346.2 psf	342.9 psf	339.8 psf	336.8 psf	417.6 psf	412.4 psf	407.6 psf	402.9 psf	453.3 psf	447.3 psf	441.5 psf	436.1 psf	104.3 psf	75.3 psf	67.5 psf	65.0 psf

Maximum Bearing Pressure = 453 psf  
Allowable Bearing Pressure = 1500 psf

Use a 132in long x 38in wide x 18in tall ballast foundation for an acceptable bearing pressure.

# Weak Side Design

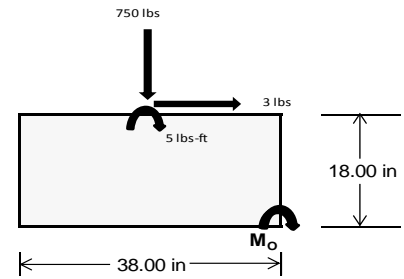
## Overturning Check

$M_o = 1177.3 \text{ ft-lbs}$   
 Resisting Force Required = 743.56 lbs  
 S.F. = 1.67  
 Weight Required = 1239.27 lbs  
 Minimum Width = 38 in  
 Weight Provided = 7576.25 lbs

*A minimum 132in long x 38in wide x 18in tall ballast foundation is required to resist overturning.*

## Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	38 in			38 in			38 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
$F_v$	213 lbs	550 lbs	213 lbs	750 lbs	2185 lbs	750 lbs	62 lbs	161 lbs	62 lbs
$F_v$	1 lbs	0 lbs	1 lbs	3 lbs	0 lbs	3 lbs	0 lbs	0 lbs	0 lbs
$P_{total}$	9593 lbs	7576 lbs	9593 lbs	9678 lbs	7576 lbs	9678 lbs	2805 lbs	7576 lbs	2805 lbs
$M$	3 lbs-ft	0 lbs-ft	3 lbs-ft	10 lbs-ft	0 lbs-ft	10 lbs-ft	0 lbs-ft	0 lbs-ft	0 lbs-ft
$e$	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft	0.53 ft
$f_{min}$	275.2 psf	217.5 psf	275.2 psf	277.3 psf	217.5 psf	277.3 psf	80.5 psf	217.5 psf	80.5 psf
$f_{max}$	275.6 psf	217.5 psf	275.6 psf	278.4 psf	217.5 psf	278.4 psf	80.5 psf	217.5 psf	80.5 psf



Maximum Bearing Pressure = 278 psf  
 Allowable Bearing Pressure = 1500 psf

*Use a 132in long x 38in wide x 18in tall ballast foundation for an acceptable bearing pressure.*

**Foundation Requirements:** 132in long x 38in wide x 18in tall ballast foundation and fiber reinforcing with (3) #5 rebar.

## 5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.



## 6. DESIGN OF JOINTS AND CONNECTIONS

### 6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of 80mm mounting clamps. The reliability of calculations is uncertain due to limited standards, therefore the strength of the clamp fasteners has been evaluated by load testing.

#### Fastening of Modules to Purlins

Maximum Uplifting Force =	0.912 k
Allowable Uplift =	1.214 k
Utilization =	<u>75%</u>



#### Fastening of Purlins to Girders

Maximum Uplifting Force =	2.771 k
Allowable Uplift =	4.357 k
Utilization =	<u>64%</u>



### 6.2 Strut Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Single M12 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

#### Front Strut

Maximum Axial Load =	3.473 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>47%</u>

#### Rear Strut

Maximum Axial Load =	4.908 k
M12 Bolt Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>66%</u>

#### Diagonal Strut

Maximum Axial Load =	2.228 k
M12 Bolt Shear Capacity =	12.808 k
Strut Bearing Capacity =	7.421 k
Utilization =	<u>30%</u>

Bolt and bearing capacities are accounting for double shear.  
(ASCE 8-02, Eq. 5.3.4-1)



Struts under compression are shown to demonstrate the load transfer from the girder. Single M12 bolts are located at each end of the strut and are subjected to double shear.

## 7. SEISMIC DESIGN

### 7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, $h_{sx}$ =	40.12 in
Allowable Story Drift for All Other Structures, $\Delta$ = {	0.020 $h_{sx}$
Max Drift, $\Delta_{MAX}$ =	0.802 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



## APPENDIX A

### A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **S1.5**

Strong Axis:

#### 3.4.14

$$L_b = 102 \text{ in}$$

$$J = 0.432$$

$$282.18$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 27.9 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 102$$

$$J = 0.432$$

$$179.449$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(lyJ)/2}))}]$$

$$\phi F_L = 29.0$$

#### 3.4.16

$$b/t = 32.195$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 25.1 \text{ ksi}$$

#### 3.4.16

$$b/t = 37.0588$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 23.1 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t =$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 37.0588$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40.985$$

$$Cc = 41.015$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.2$$

$$\phi F_L = \phi b [Bbr - mDbr \cdot h/t]$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 25.1 \text{ ksi}$$

$$I_x = 897074 \text{ mm}^4$$

$$2.155 \text{ in}^4$$

$$y = 41.015 \text{ mm}$$

$$S_x = 1.335 \text{ in}^3$$

$$M_{\max} St = 2.788 \text{ k-ft}$$

#### 3.4.18

$$h/t = 32.195$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 45.5$$

$$Cc = 45.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 23.1 \text{ ksi}$$

$$I_y = 446476 \text{ mm}^4$$

$$1.073 \text{ in}^4$$

$$x = 45.5 \text{ mm}$$

$$S_y = 0.599 \text{ in}^3$$

$$M_{\max} Wk = 1.152 \text{ k-ft}$$

## Compression

### 3.4.9

$$\begin{aligned} b/t &= 32.195 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 25.1 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 37.0588 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= (\phi c k_2 \cdot \sqrt{(BpE)}) / (1.6b/t) \\ \phi F_L &= 21.9 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 21.94 \text{ ksi} \\ A &= 1215.13 \text{ mm}^2 \\ &= 1.88 \text{ in}^2 \\ P_{\max} &= 41.32 \text{ kips} \end{aligned}$$

## A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **BF0**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \text{ in} \\ J &= 1.08 \\ &= 152.913 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.4 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 16.2 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 31.6 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 88.9 \\ J &= 1.08 \\ &= 161.829 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 29.2 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 7.4 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.3 \text{ ksi} \end{aligned}$$

### 3.4.16.1 Used

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = \phi b [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.1 \text{ ksi}$$

### 3.4.18

$$h/t = 7.4$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 35.2$$

$$m = 0.68$$

$$C_0 = 41.067$$

$$Cc = 43.717$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 73.8$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.4 \text{ ksi}$$

$$I_x = 984962 \text{ mm}^4$$

$$2.366 \text{ in}^4$$

$$y = 43.717 \text{ mm}$$

$$S_x = 1.375 \text{ in}^3$$

$$M_{max} St = 3.363 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 16.2$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 40$$

$$Cc = 40$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3\phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 923544 \text{ mm}^4$$

$$2.219 \text{ in}^4$$

$$x = 40 \text{ mm}$$

$$S_y = 1.409 \text{ in}^3$$

$$M_{max} Wk = 3.904 \text{ k-ft}$$

### Compression

### 3.4.9

$$b/t = 16.2$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi c [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 31.6 \text{ ksi}$$

$$b/t = 7.4$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

### 3.4.10

$$Rb/t = 18.1$$

$$S1 = \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi F_L = \phi c [Bt - Dt \sqrt{(Rb/t)}]$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$\phi F_L = 31.09 \text{ ksi}$$

$$A = 1215.13 \text{ mm}^2$$

$$1.88 \text{ in}^2$$

$$P_{max} = 58.55 \text{ kips}$$

### A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

#### 3.4.14

$$L_b = 24.8 \text{ in}$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4 \text{ ksi}$$

Weak Axis:

#### 3.4.14

$$L_b = 24.8$$

$$J = \frac{0.942}{38.7028}$$

$$S1 = \left( \frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left( \frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.4$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

## Compression

### 3.4.7

$$\begin{aligned}\lambda &= 0.57371 \\ r &= 0.81 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.87952 \\ \phi_{FL} &= \phi_{cc}(Bc - Dc^*\lambda) \\ \phi_{FL} &= 28.0279 \text{ ksi}\end{aligned}$$

### 3.4.9

$$\begin{aligned}b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi} \\ b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_c[Bp - 1.6Dp*b/t] \\ \phi_{FL} &= 28.2 \text{ ksi}\end{aligned}$$

### 3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 28.03 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 28.85 \text{ kips}\end{aligned}$$

## A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.60 \text{ in} \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6 \text{ ksi}\end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned}L_b &= 86.6 \\ J &= 0.942 \\ &= 135.148 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{Cc}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi_{FL} &= \phi_b[Bc - 1.6Dc^*\sqrt{((LbSc)/(Cb^*\sqrt{(IyJ)/2}))}] \\ \phi_{FL} &= 29.6\end{aligned}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

#### Compression

### 3.4.7

$$\lambda = 2.00335$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.86047$$

$$\phi F_L = (\phi_{cc} Fcy) / (\lambda^2)$$

$$\phi F_L = 7.50396 \text{ ksi}$$

### 3.4.16

$$b/t = 24.5$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

#### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### 3.4.9

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

$$\begin{aligned} b/t &= 24.5 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi c [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi y Fcy \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 7.50 \text{ ksi} \\ A &= 663.99 \text{ mm}^2 \\ &= 1.03 \text{ in}^2 \\ P_{\max} &= 7.72 \text{ kips} \end{aligned}$$

## A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **55x55**

Strong Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \text{ in} \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \text{ ksi} \end{aligned}$$

Weak Axis:

### 3.4.14

$$\begin{aligned} L_b &= 55.91 \\ J &= 0.942 \\ &= 87.2529 \\ S1 &= \left( \frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2 \\ S1 &= 0.51461 \\ S2 &= \left( \frac{C_c}{1.6} \right)^2 \\ S2 &= 1701.56 \\ \phi F_L &= \phi b [Bc - 1.6Dc \cdot \sqrt{((LbSc)/(Cb \cdot \sqrt{(IyJ)/2}))}] \\ \phi F_L &= 30.4 \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

### 3.4.16

$$\begin{aligned} b/t &= 24.5 \\ S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\ S1 &= 12.2 \\ S2 &= \frac{k_1 Bp}{1.6Dp} \\ S2 &= 46.7 \\ \phi F_L &= \phi b [Bp - 1.6Dp \cdot b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$



### 3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left( \frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 28.2 \text{ ksi}$$

$$I_x = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$y = 27.5 \text{ mm}$$

$$S_x = 0.621 \text{ in}^3$$

$$M_{\max} St = 1.460 \text{ k-ft}$$

### 3.4.16.1

N/A for Weak Direction

### 3.4.18

$$h/t = 24.5$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 27.5$$

$$Cc = 27.5$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 28.2 \text{ ksi}$$

$$I_y = 279836 \text{ mm}^4$$

$$0.672 \text{ in}^4$$

$$x = 27.5 \text{ mm}$$

$$S_y = 0.621 \text{ in}^3$$

$$M_{\max} Wk = 1.460 \text{ k-ft}$$

### Compression

### 3.4.7

$$\lambda = 1.29339$$

$$r = 0.81 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.76107$$

$$\phi F_L = (\phi_{cc} Fcy)/(\lambda^2)$$

$$\phi F_L = 15.9235 \text{ ksi}$$

### 3.4.9

$$b/t = 24.5$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

$$b/t = 24.5$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi F_L = \phi_c [Bp - 1.6Dp^* b/t]$$

$$\phi F_L = 28.2 \text{ ksi}$$

### 3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left( \frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \\ 
 \phi F_L &= 15.92 \text{ ksi} \\
 A &= 663.99 \text{ mm}^2 \\
 &= 1.03 \text{ in}^2 \\
 P_{\max} &= 16.39 \text{ kips}
 \end{aligned}$$

## APPENDIX B

### B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Basic Load Cases

	BLC Description	Category	X Gravity	Y Gravity	Z Gravity	Joint	Point	Distribut...	Area(Me...	Surface(...
1	Dead Load, Max	DL		-1				4		
2	Dead Load, Min	DL		-1				4		
3	Snow Load	SL						4		
4	Wind Load - Pressure	WL						4		
5	Wind Load - Suction	WL						4		
6	Seismic - Lateral	EL								

### Member Distributed Loads (BLC 1 : Dead Load, Max)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-8.366	-8.366	0	0
2	M14	Y	-8.366	-8.366	0	0
3	M15	Y	-8.366	-8.366	0	0
4	M16	Y	-8.366	-8.366	0	0

### Member Distributed Loads (BLC 2 : Dead Load, Min)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-4.45	-4.45	0	0
2	M14	Y	-4.45	-4.45	0	0
3	M15	Y	-4.45	-4.45	0	0
4	M16	Y	-4.45	-4.45	0	0

### Member Distributed Loads (BLC 3 : Snow Load)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	Y	-54.031	-54.031	0	0
2	M14	Y	-54.031	-54.031	0	0
3	M15	Y	-54.031	-54.031	0	0
4	M16	Y	-54.031	-54.031	0	0

### Member Distributed Loads (BLC 4 : Wind Load - Pressure)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	-117.695	-117.695	0	0
2	M14	y	-117.695	-117.695	0	0
3	M15	y	-184.95	-184.95	0	0
4	M16	y	-184.95	-184.95	0	0

### Member Distributed Loads (BLC 5 : Wind Load - Suction)

	Member Label	Direction	Start Magnitude[lb/ft,F]	End Magnitude[lb/ft,F]	Start Location[ft, %]	End Location[ft, %]
1	M13	y	269.018	269.018	0	0
2	M14	y	206.247	206.247	0	0
3	M15	y	112.091	112.091	0	0
4	M16	y	112.091	112.091	0	0

### Load Combinations

	Description	S... P...	S... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...	Fa... B...
1	LRFD 1.2D + 1.6S + 0.5W	Yes Y		1	1.2	3	1.6	4	.5										
2	LRFD 1.2D + 1.0W + 0.5S	Yes Y		1	1.2	3	.5	4	1										
3	LRFD 0.9D + 1.0W	Yes Y		2	.9					5	1								
4	LATERAL - LRFD 1.54D + 1.3E ...	Yes Y		1	1.54	3	.2			6	1.3								
5	LATERAL - LRFD 0.56D + 1.3E	Yes Y		1	.56					6	1.3								
6	LATERAL - LRFD 1.54D + 1.25...	Yes Y		1	1.54	3	.2			6	1.25								
7	LATERAL - LRFD 0.56D + 1.25E	Yes Y		1	.56					6	1.25								



RISA-3D Version 13.0.0 [T:\...\PVMMax 60 Cell 2V 20° 160mph 30psf 8.5ft 7-10 NS.r3d] Page 19



Company : Schletter, Inc.  
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Job Number :  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
27		14	max	69.537	1	245.633	2	.519	3	.016	2	-.004	15	1.033	3
28			min	2.567	15	-404.495	3	-22.621	1	0	15	-.103	1	-.559	2
29		15	max	69.537	1	99.989	2	9.13	1	.016	2	-.004	12	1.297	3
30			min	2.567	15	-155.179	3	.234	10	0	15	-.109	1	-.723	2
31		16	max	69.537	1	94.138	3	40.881	1	.016	2	-.002	12	1.326	3
32			min	2.567	15	-45.842	1	1.509	15	0	15	-.086	1	-.748	2
33		17	max	69.537	1	343.455	3	72.632	1	.016	2	.002	3	1.12	3
34			min	2.567	15	-191.299	2	2.663	15	0	15	-.032	1	-.636	2
35		18	max	69.537	1	592.772	3	104.382	1	.016	2	.052	1	.678	3
36			min	2.567	15	-336.943	2	3.817	15	0	15	.002	15	-.387	2
37		19	max	69.537	1	842.088	3	136.133	1	.016	2	.165	1	0	1
38			min	2.567	15	-482.587	2	4.971	15	0	15	.006	15	0	3
39	M14	1	max	35.785	1	530.587	2	-5.147	15	.012	3	.192	1	0	1
40			min	1.32	15	-670.426	3	-140.945	1	-.014	2	.007	15	0	3
41		2	max	35.785	1	384.943	2	-3.993	15	.012	3	.074	1	.543	3
42			min	1.32	15	-480.393	3	-109.194	1	-.014	2	.003	15	-.432	2
43		3	max	35.785	1	239.299	2	-2.838	15	.012	3	.003	3	.907	3
44			min	1.32	15	-290.36	3	-77.444	1	-.014	2	-.014	1	-.727	2
45		4	max	35.785	1	93.656	2	-1.684	15	.012	3	-.001	12	1.092	3
46			min	1.32	15	-100.327	3	-45.693	1	-.014	2	-.072	1	-.884	2
47		5	max	35.785	1	89.706	3	-.53	15	.012	3	-.003	12	1.097	3
48			min	1.32	15	-53.91	1	-13.942	1	-.014	2	-.1	1	-.904	2
49		6	max	35.785	1	279.739	3	17.809	1	.012	3	-.004	15	.922	3
50			min	1.32	15	-197.632	2	-.787	3	-.014	2	-.098	1	-.786	2
51		7	max	35.785	1	469.772	3	49.56	1	.012	3	-.002	15	.569	3
52			min	1.32	15	-343.276	2	.743	12	-.014	2	-.067	1	-.531	2
53		8	max	35.785	1	659.805	3	81.31	1	.012	3	.001	10	.035	3
54			min	1.32	15	-488.92	2	1.897	12	-.014	2	-.005	1	-.138	2
55		9	max	35.785	1	849.838	3	113.061	1	.012	3	.087	1	.407	1
56			min	1.32	15	-634.564	2	3.051	12	-.014	2	-.001	3	-.678	3
57		10	max	35.785	1	1039.871	3	144.812	1	.014	2	.209	1	1.065	1
58			min	1.32	15	-780.208	2	4.205	12	-.012	3	.003	12	-1.57	3
59		11	max	35.785	1	634.564	2	-3.051	12	.014	2	.087	1	.407	1
60			min	1.32	15	-849.838	3	-113.061	1	-.012	3	-.001	3	-.678	3
61		12	max	35.785	1	488.92	2	-1.897	12	.014	2	.001	10	.035	3
62			min	1.32	15	-659.805	3	-81.31	1	-.012	3	-.005	1	-.138	2
63		13	max	35.785	1	343.276	2	-.743	12	.014	2	-.002	15	.569	3
64			min	1.32	15	-469.772	3	-49.56	1	-.012	3	-.067	1	-.531	2
65		14	max	35.785	1	197.632	2	.787	3	.014	2	-.004	15	.922	3
66			min	1.32	15	-279.739	3	-17.809	1	-.012	3	-.098	1	-.786	2
67		15	max	35.785	1	53.91	1	13.942	1	.014	2	-.003	12	1.097	3
68			min	1.32	15	-89.706	3	.53	15	-.012	3	-.1	1	-.904	2
69		16	max	35.785	1	100.327	3	45.693	1	.014	2	-.001	12	1.092	3
70			min	1.32	15	-93.656	2	1.684	15	-.012	3	-.072	1	-.884	2
71		17	max	35.785	1	290.36	3	77.444	1	.014	2	.003	3	.907	3
72			min	1.32	15	-239.299	2	2.838	15	-.012	3	-.014	1	-.727	2
73		18	max	35.785	1	480.393	3	109.194	1	.014	2	.074	1	.543	3
74			min	1.32	15	-384.943	2	3.993	15	-.012	3	.003	15	-.432	2
75		19	max	35.785	1	670.426	3	140.945	1	.014	2	.192	1	0	1
76			min	1.32	15	-530.587	2	5.147	15	-.012	3	.007	15	0	3
77	M15	1	max	-1.38	15	745.057	2	-5.145	15	.014	2	.192	1	0	2
78			min	-37.27	1	-370.763	3	-140.947	1	-.01	3	.007	15	0	3
79		2	max	-1.38	15	535.895	2	-3.991	15	.014	2	.074	1	.302	3
80			min	-37.27	1	-269.656	3	-109.196	1	-.01	3	.003	15	-.605	2
81		3	max	-1.38	15	326.732	2	-2.837	15	.014	2	.003	3	.509	3
82			min	-37.27	1	-168.548	3	-77.445	1	-.01	3	-.014	1	-1.012	2
83		4	max	-1.38	15	117.57	2	-1.683	15	.014	2	-.001	12	.621	3



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
84			min	-37.27	1	-67.44	3	-45.694	1	-.01	3	-.072	1	-1.222	2
85		5	max	-1.38	15	33.668	3	-.529	15	.014	2	-.003	12	.637	3
86			min	-37.27	1	-91.593	2	-13.944	1	-.01	3	-.1	1	-1.234	2
87		6	max	-1.38	15	134.776	3	17.807	1	.014	2	-.004	15	.557	3
88			min	-37.27	1	-300.755	2	-.654	3	-.01	3	-.098	1	-1.049	2
89		7	max	-1.38	15	235.884	3	49.558	1	.014	2	-.002	15	.382	3
90			min	-37.27	1	-509.918	2	.823	12	-.01	3	-.067	1	-.666	2
91		8	max	-1.38	15	336.992	3	81.309	1	.014	2	.001	10	.112	3
92			min	-37.27	1	-719.08	2	1.977	12	-.01	3	-.005	1	-.089	1
93		9	max	-1.38	15	438.1	3	113.059	1	.014	2	.087	1	.692	2
94			min	-37.27	1	-928.243	2	3.131	12	-.01	3	0	3	-.254	3
95		10	max	-1.38	15	539.208	3	144.81	1	.01	3	.209	1	1.667	2
96			min	-37.27	1	-1137.405	2	4.285	12	-.014	2	.003	12	-.716	3
97		11	max	-1.38	15	928.243	2	-3.131	12	.01	3	.087	1	.692	2
98			min	-37.27	1	-438.1	3	-113.059	1	-.014	2	0	3	-.254	3
99		12	max	-1.38	15	719.08	2	-1.977	12	.01	3	.001	10	.112	3
100			min	-37.27	1	-336.992	3	-81.309	1	-.014	2	-.005	1	-.089	1
101		13	max	-1.38	15	509.918	2	-.823	12	.01	3	-.002	15	.382	3
102			min	-37.27	1	-235.884	3	-49.558	1	-.014	2	-.067	1	-.666	2
103		14	max	-1.38	15	300.755	2	.654	3	.01	3	-.004	15	.557	3
104			min	-37.27	1	-134.776	3	-17.807	1	-.014	2	-.098	1	-1.049	2
105		15	max	-1.38	15	91.593	2	13.944	1	.01	3	-.003	12	.637	3
106			min	-37.27	1	-33.668	3	.529	15	-.014	2	-.1	1	-1.234	2
107		16	max	-1.38	15	67.44	3	45.694	1	.01	3	-.001	12	.621	3
108			min	-37.27	1	-117.57	2	1.683	15	-.014	2	-.072	1	-1.222	2
109		17	max	-1.38	15	168.548	3	77.445	1	.01	3	.003	3	.509	3
110			min	-37.27	1	-326.732	2	2.837	15	-.014	2	-.014	1	-1.012	2
111		18	max	-1.38	15	269.656	3	109.196	1	.01	3	.074	1	.302	3
112			min	-37.27	1	-535.895	2	3.991	15	-.014	2	.003	15	-.605	2
113		19	max	-1.38	15	370.763	3	140.947	1	.01	3	.192	1	0	2
114			min	-37.27	1	-745.057	2	5.145	15	-.014	2	.007	15	0	3
115	M16	1	max	-2.734	15	699.049	2	-4.977	15	.011	1	.167	1	0	2
116			min	-74.105	1	-333.417	3	-136.416	1	-.013	3	.006	15	0	3
117		2	max	-2.734	15	489.886	2	-3.823	15	.011	1	.053	1	.267	3
118			min	-74.105	1	-232.309	3	-104.666	1	-.013	3	.002	15	-.561	2
119		3	max	-2.734	15	280.724	2	-2.669	15	.011	1	.001	3	.439	3
120			min	-74.105	1	-131.201	3	-72.915	1	-.013	3	-.031	1	-.925	2
121		4	max	-2.734	15	71.562	2	-1.515	15	.011	1	-.002	12	.515	3
122			min	-74.105	1	-30.094	3	-41.164	1	-.013	3	-.085	1	-1.092	2
123		5	max	-2.734	15	71.014	3	-.36	15	.011	1	-.004	12	.496	3
124			min	-74.105	1	-137.601	2	-9.413	1	-.013	3	-.109	1	-1.061	2
125		6	max	-2.734	15	172.122	3	22.337	1	.011	1	-.004	15	.381	3
126			min	-74.105	1	-346.763	2	-.07	3	-.013	3	-.103	1	-.832	2
127		7	max	-2.734	15	273.23	3	54.088	1	.011	1	-.002	15	.171	3
128			min	-74.105	1	-555.926	2	1.191	12	-.013	3	-.067	1	-.406	2
129		8	max	-2.734	15	374.338	3	85.839	1	.011	1	.002	2	.218	2
130			min	-74.105	1	-765.088	2	2.345	12	-.013	3	-.003	3	-.135	3
131		9	max	-2.734	15	475.446	3	117.59	1	.011	1	.095	1	1.04	2
132			min	-74.105	1	-974.251	2	3.499	12	-.013	3	0	3	-.537	3
133		10	max	-2.734	15	576.554	3	149.34	1	.013	3	.222	1	2.059	2
134			min	-74.105	1	-1183.413	2	4.652	12	-.011	1	.005	12	-1.033	3
135		11	max	-2.734	15	974.251	2	-3.499	12	.013	3	.095	1	1.04	2
136			min	-74.105	1	-475.446	3	-117.59	1	-.011	1	0	3	-.537	3
137		12	max	-2.734	15	765.088	2	-2.345	12	.013	3	.002	2	.218	2
138			min	-74.105	1	-374.338	3	-85.839	1	-.011	1	-.003	3	-.135	3
139		13	max	-2.734	15	555.926	2	-1.191	12	.013	3	-.002	15	.171	3
140			min	-74.105	1	-273.23	3	-54.088	1	-.011	1	-.067	1	-.406	2



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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
141		14	max	-2.734	15	346.763	2	.07	3	.013	3	-.004	15	.381	3
142			min	-74.105	1	-172.122	3	-22.337	1	-.011	1	-.103	1	-.832	2
143		15	max	-2.734	15	137.601	2	9.413	1	.013	3	-.004	12	.496	3
144			min	-74.105	1	-71.014	3	.36	15	-.011	1	-.109	1	-1.061	2
145		16	max	-2.734	15	30.094	3	41.164	1	.013	3	-.002	12	.515	3
146			min	-74.105	1	-71.562	2	1.515	15	-.011	1	-.085	1	-1.092	2
147		17	max	-2.734	15	131.201	3	72.915	1	.013	3	.001	3	.439	3
148			min	-74.105	1	-280.724	2	2.669	15	-.011	1	-.031	1	-.925	2
149		18	max	-2.734	15	232.309	3	104.666	1	.013	3	.053	1	.267	3
150			min	-74.105	1	-489.886	2	3.823	15	-.011	1	.002	15	-.561	2
151		19	max	-2.734	15	333.417	3	136.416	1	.013	3	.167	1	0	2
152			min	-74.105	1	-699.049	2	4.977	15	-.011	1	.006	15	0	3
153	M2	1	max	1120.731	2	2.029	4	.651	1	0	3	0	3	0	1
154			min	-1520.659	3	.478	15	.024	15	0	1	0	2	0	1
155		2	max	1121.111	2	1.995	4	.651	1	0	3	0	1	0	15
156			min	-1520.375	3	.47	15	.024	15	0	1	0	15	0	4
157		3	max	1121.49	2	1.962	4	.651	1	0	3	0	1	0	15
158			min	-1520.09	3	.462	15	.024	15	0	1	0	15	-.001	4
159		4	max	1121.869	2	1.928	4	.651	1	0	3	0	1	0	15
160			min	-1519.806	3	.454	15	.024	15	0	1	0	15	-.002	4
161		5	max	1122.248	2	1.895	4	.651	1	0	3	0	1	0	15
162			min	-1519.521	3	.446	15	.024	15	0	1	0	15	-.002	4
163		6	max	1122.628	2	1.862	4	.651	1	0	3	0	1	0	15
164			min	-1519.237	3	.438	15	.024	15	0	1	0	15	-.002	4
165		7	max	1123.007	2	1.828	4	.651	1	0	3	0	1	0	15
166			min	-1518.952	3	.431	15	.024	15	0	1	0	15	-.003	4
167		8	max	1123.386	2	1.795	4	.651	1	0	3	.001	1	0	15
168			min	-1518.668	3	.423	15	.024	15	0	1	0	15	-.003	4
169		9	max	1123.765	2	1.761	4	.651	1	0	3	.001	1	0	15
170			min	-1518.384	3	.415	15	.024	15	0	1	0	15	-.004	4
171		10	max	1124.145	2	1.728	4	.651	1	0	3	.001	1	-.001	15
172			min	-1518.099	3	.407	15	.024	15	0	1	0	15	-.004	4
173		11	max	1124.524	2	1.695	4	.651	1	0	3	.002	1	-.001	15
174			min	-1517.815	3	.398	12	.024	15	0	1	0	15	-.005	4
175		12	max	1124.903	2	1.661	4	.651	1	0	3	.002	1	-.001	15
176			min	-1517.53	3	.385	12	.024	15	0	1	0	15	-.005	4
177		13	max	1125.282	2	1.628	4	.651	1	0	3	.002	1	-.001	15
178			min	-1517.246	3	.372	12	.024	15	0	1	0	15	-.006	4
179		14	max	1125.662	2	1.594	4	.651	1	0	3	.002	1	-.001	15
180			min	-1516.961	3	.359	12	.024	15	0	1	0	15	-.006	4
181		15	max	1126.041	2	1.561	4	.651	1	0	3	.002	1	-.002	15
182			min	-1516.677	3	.346	12	.024	15	0	1	0	15	-.006	4
183		16	max	1126.42	2	1.528	4	.651	1	0	3	.002	1	-.002	15
184			min	-1516.392	3	.333	12	.024	15	0	1	0	15	-.007	4
185		17	max	1126.799	2	1.494	4	.651	1	0	3	.003	1	-.002	15
186			min	-1516.108	3	.32	12	.024	15	0	1	0	15	-.007	4
187		18	max	1127.179	2	1.461	2	.651	1	0	3	.003	1	-.002	15
188			min	-1515.824	3	.307	12	.024	15	0	1	0	15	-.008	4
189		19	max	1127.558	2	1.435	2	.651	1	0	3	.003	1	-.002	15
190			min	-1515.539	3	.294	12	.024	15	0	1	0	15	-.008	4
191	M3	1	max	583.7	2	7.983	4	.066	1	0	3	0	1	.008	4
192			min	-714.858	3	1.877	15	.002	15	0	1	0	15	.002	15
193		2	max	583.53	2	7.213	4	.066	1	0	3	0	1	.005	2
194			min	-714.985	3	1.696	15	.002	15	0	1	0	15	0	12
195		3	max	583.36	2	6.443	4	.066	1	0	3	0	1	.003	2
196			min	-715.113	3	1.515	15	.002	15	0	1	0	15	0	3
197		4	max	583.189	2	5.673	4	.066	1	0	3	0	1	0	2





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
198		min	-715.241	3	1.334	15	.002	15	0	1	0	15	-.002	3
199	5	max	583.019	2	4.903	4	.066	1	0	3	0	1	0	15
200		min	-715.369	3	1.153	15	.002	15	0	1	0	15	-.003	3
201	6	max	582.849	2	4.133	4	.066	1	0	3	0	1	-.001	15
202		min	-715.497	3	.972	15	.002	15	0	1	0	15	-.005	4
203	7	max	582.678	2	3.363	4	.066	1	0	3	0	1	-.001	15
204		min	-715.624	3	.791	15	.002	15	0	1	0	15	-.006	4
205	8	max	582.508	2	2.593	4	.066	1	0	3	0	1	-.002	15
206		min	-715.752	3	.61	15	.002	15	0	1	0	15	-.008	4
207	9	max	582.338	2	1.823	4	.066	1	0	3	0	1	-.002	15
208		min	-715.88	3	.429	15	.002	15	0	1	0	15	-.009	4
209	10	max	582.167	2	1.053	4	.066	1	0	3	0	1	-.002	15
210		min	-716.008	3	.248	15	.002	15	0	1	0	15	-.009	4
211	11	max	581.997	2	.417	2	.066	1	0	3	0	1	-.002	15
212		min	-716.135	3	-.101	3	.002	15	0	1	0	15	-.009	4
213	12	max	581.827	2	-.114	15	.066	1	0	3	0	1	-.002	15
214		min	-716.263	3	-.551	3	.002	15	0	1	0	15	-.009	4
215	13	max	581.656	2	-.295	15	.066	1	0	3	0	1	-.002	15
216		min	-716.391	3	-1.257	4	.002	15	0	1	0	15	-.009	4
217	14	max	581.486	2	-.476	15	.066	1	0	3	0	1	-.002	15
218		min	-716.519	3	-2.027	4	.002	15	0	1	0	15	-.008	4
219	15	max	581.316	2	-.657	15	.066	1	0	3	0	1	-.002	15
220		min	-716.646	3	-2.797	4	.002	15	0	1	0	15	-.007	4
221	16	max	581.145	2	-.838	15	.066	1	0	3	0	1	-.001	15
222		min	-716.774	3	-3.567	4	.002	15	0	1	0	15	-.006	4
223	17	max	580.975	2	-1.019	15	.066	1	0	3	0	1	-.001	15
224		min	-716.902	3	-4.337	4	.002	15	0	1	0	15	-.004	4
225	18	max	580.805	2	-1.2	15	.066	1	0	3	0	1	0	15
226		min	-717.03	3	-5.107	4	.002	15	0	1	0	15	-.002	4
227	19	max	580.634	2	-1.381	15	.066	1	0	3	0	1	0	1
228		min	-717.157	3	-5.877	4	.002	15	0	1	0	15	0	1
229	M4	1	max	1172.358	1	0	1	-.254	15	0	1	0	1	0
230		min	-334.59	3	0	1	-6.92	1	0	1	0	15	0	1
231	2	max	1172.528	1	0	1	-.254	15	0	1	0	12	0	1
232		min	-334.462	3	0	1	-6.92	1	0	1	0	1	0	1
233	3	max	1172.699	1	0	1	-.254	15	0	1	0	15	0	1
234		min	-334.334	3	0	1	-6.92	1	0	1	-.001	1	0	1
235	4	max	1172.869	1	0	1	-.254	15	0	1	0	15	0	1
236		min	-334.206	3	0	1	-6.92	1	0	1	-.002	1	0	1
237	5	max	1173.039	1	0	1	-.254	15	0	1	0	15	0	1
238		min	-334.079	3	0	1	-6.92	1	0	1	-.003	1	0	1
239	6	max	1173.21	1	0	1	-.254	15	0	1	0	15	0	1
240		min	-333.951	3	0	1	-6.92	1	0	1	-.004	1	0	1
241	7	max	1173.38	1	0	1	-.254	15	0	1	0	15	0	1
242		min	-333.823	3	0	1	-6.92	1	0	1	-.004	1	0	1
243	8	max	1173.55	1	0	1	-.254	15	0	1	0	15	0	1
244		min	-333.695	3	0	1	-6.92	1	0	1	-.005	1	0	1
245	9	max	1173.721	1	0	1	-.254	15	0	1	0	15	0	1
246		min	-333.568	3	0	1	-6.92	1	0	1	-.006	1	0	1
247	10	max	1173.891	1	0	1	-.254	15	0	1	0	15	0	1
248		min	-333.44	3	0	1	-6.92	1	0	1	-.007	1	0	1
249	11	max	1174.061	1	0	1	-.254	15	0	1	0	15	0	1
250		min	-333.312	3	0	1	-6.92	1	0	1	-.008	1	0	1
251	12	max	1174.232	1	0	1	-.254	15	0	1	0	15	0	1
252		min	-333.184	3	0	1	-6.92	1	0	1	-.008	1	0	1
253	13	max	1174.402	1	0	1	-.254	15	0	1	0	15	0	1
254		min	-333.057	3	0	1	-6.92	1	0	1	-.009	1	0	1





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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
255	14	max	1174.573	1	0	1	-.254	15	0	1	0	15	0	1
256		min	-332.929	3	0	1	-6.92	1	0	1	-.01	1	0	1
257	15	max	1174.743	1	0	1	-.254	15	0	1	0	15	0	1
258		min	-332.801	3	0	1	-6.92	1	0	1	-.011	1	0	1
259	16	max	1174.913	1	0	1	-.254	15	0	1	0	15	0	1
260		min	-332.673	3	0	1	-6.92	1	0	1	-.011	1	0	1
261	17	max	1175.084	1	0	1	-.254	15	0	1	0	15	0	1
262		min	-332.546	3	0	1	-6.92	1	0	1	-.012	1	0	1
263	18	max	1175.254	1	0	1	-.254	15	0	1	0	15	0	1
264		min	-332.418	3	0	1	-6.92	1	0	1	-.013	1	0	1
265	19	max	1175.424	1	0	1	-.254	15	0	1	0	15	0	1
266		min	-332.29	3	0	1	-6.92	1	0	1	-.014	1	0	1
267	M6	1	max	3545.989	2	2.692	2	0	1	0	0	1	0	1
268		min	-4908.388	3	-.199	3	0	1	0	1	0	1	0	1
269	2	max	3546.368	2	2.666	2	0	1	0	1	0	1	0	3
270		min	-4908.104	3	-.218	3	0	1	0	1	0	1	0	2
271	3	max	3546.747	2	2.64	2	0	1	0	1	0	1	0	3
272		min	-4907.819	3	-.238	3	0	1	0	1	0	1	-.001	2
273	4	max	3547.127	2	2.614	2	0	1	0	1	0	1	0	3
274		min	-4907.535	3	-.257	3	0	1	0	1	0	1	-.002	2
275	5	max	3547.506	2	2.588	2	0	1	0	1	0	1	0	3
276		min	-4907.251	3	-.277	3	0	1	0	1	0	1	-.003	2
277	6	max	3547.885	2	2.562	2	0	1	0	1	0	1	0	3
278		min	-4906.966	3	-.296	3	0	1	0	1	0	1	-.003	2
279	7	max	3548.264	2	2.536	2	0	1	0	1	0	1	0	3
280		min	-4906.682	3	-.316	3	0	1	0	1	0	1	-.004	2
281	8	max	3548.644	2	2.51	2	0	1	0	1	0	1	0	3
282		min	-4906.397	3	-.335	3	0	1	0	1	0	1	-.005	2
283	9	max	3549.023	2	2.484	2	0	1	0	1	0	1	0	3
284		min	-4906.113	3	-.355	3	0	1	0	1	0	1	-.005	2
285	10	max	3549.402	2	2.458	2	0	1	0	1	0	1	0	3
286		min	-4905.828	3	-.374	3	0	1	0	1	0	1	-.006	2
287	11	max	3549.781	2	2.432	2	0	1	0	1	0	1	0	3
288		min	-4905.544	3	-.394	3	0	1	0	1	0	1	-.007	2
289	12	max	3550.161	2	2.406	2	0	1	0	1	0	1	0	3
290		min	-4905.259	3	-.413	3	0	1	0	1	0	1	-.007	2
291	13	max	3550.54	2	2.38	2	0	1	0	1	0	1	0	3
292		min	-4904.975	3	-.433	3	0	1	0	1	0	1	-.008	2
293	14	max	3550.919	2	2.354	2	0	1	0	1	0	1	.001	3
294		min	-4904.691	3	-.452	3	0	1	0	1	0	1	-.008	2
295	15	max	3551.299	2	2.328	2	0	1	0	1	0	1	.001	3
296		min	-4904.406	3	-.472	3	0	1	0	1	0	1	-.009	2
297	16	max	3551.678	2	2.302	2	0	1	0	1	0	1	.001	3
298		min	-4904.122	3	-.491	3	0	1	0	1	0	1	-.01	2
299	17	max	3552.057	2	2.276	2	0	1	0	1	0	1	.001	3
300		min	-4903.837	3	-.511	3	0	1	0	1	0	1	-.01	2
301	18	max	3552.436	2	2.25	2	0	1	0	1	0	1	.002	3
302		min	-4903.553	3	-.53	3	0	1	0	1	0	1	-.011	2
303	19	max	3552.816	2	2.224	2	0	1	0	1	0	1	.002	3
304		min	-4903.268	3	-.55	3	0	1	0	1	0	1	-.011	2
305	M7	1	max	2082.665	2	8.014	4	0	1	0	0	1	.011	2
306		min	-2225.257	3	1.881	15	0	1	0	1	0	1	-.002	3
307	2	max	2082.495	2	7.244	4	0	1	0	1	0	1	.009	2
308		min	-2225.385	3	1.7	15	0	1	0	1	0	1	-.003	3
309	3	max	2082.325	2	6.474	4	0	1	0	1	0	1	.006	2
310		min	-2225.512	3	1.519	15	0	1	0	1	0	1	-.005	3
311	4	max	2082.154	2	5.704	4	0	1	0	1	0	1	.004	2



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
312		min	-2225.64	3	1.338	15	0	1	0	1	0	1	-.006	3
313	5	max	2081.984	2	4.934	4	0	1	0	1	0	1	.002	2
314		min	-2225.768	3	1.157	15	0	1	0	1	0	1	-.007	3
315	6	max	2081.814	2	4.164	4	0	1	0	1	0	1	0	2
316		min	-2225.896	3	.976	15	0	1	0	1	0	1	-.007	3
317	7	max	2081.643	2	3.394	4	0	1	0	1	0	1	-.001	15
318		min	-2226.023	3	.795	15	0	1	0	1	0	1	-.008	3
319	8	max	2081.473	2	2.698	2	0	1	0	1	0	1	-.002	15
320		min	-2226.151	3	.504	12	0	1	0	1	0	1	-.008	3
321	9	max	2081.303	2	2.098	2	0	1	0	1	0	1	-.002	15
322		min	-2226.279	3	.204	12	0	1	0	1	0	1	-.009	3
323	10	max	2081.132	2	1.498	2	0	1	0	1	0	1	-.002	15
324		min	-2226.407	3	-.228	3	0	1	0	1	0	1	-.009	4
325	11	max	2080.962	2	.898	2	0	1	0	1	0	1	-.002	15
326		min	-2226.534	3	-.678	3	0	1	0	1	0	1	-.009	4
327	12	max	2080.791	2	.298	2	0	1	0	1	0	1	-.002	15
328		min	-2226.662	3	-1.128	3	0	1	0	1	0	1	-.009	4
329	13	max	2080.621	2	-.291	15	0	1	0	1	0	1	-.002	15
330		min	-2226.79	3	-1.578	3	0	1	0	1	0	1	-.009	4
331	14	max	2080.451	2	-.472	15	0	1	0	1	0	1	-.002	15
332		min	-2226.918	3	-2.028	3	0	1	0	1	0	1	-.008	4
333	15	max	2080.28	2	-.653	15	0	1	0	1	0	1	-.002	15
334		min	-2227.045	3	-2.765	4	0	1	0	1	0	1	-.007	4
335	16	max	2080.11	2	-.834	15	0	1	0	1	0	1	-.001	15
336		min	-2227.173	3	-3.535	4	0	1	0	1	0	1	-.006	4
337	17	max	2079.94	2	-1.015	15	0	1	0	1	0	1	-.001	15
338		min	-2227.301	3	-4.305	4	0	1	0	1	0	1	-.004	4
339	18	max	2079.769	2	-1.196	15	0	1	0	1	0	1	0	15
340		min	-2227.429	3	-5.075	4	0	1	0	1	0	1	-.002	4
341	19	max	2079.599	2	-1.377	15	0	1	0	1	0	1	0	1
342		min	-2227.556	3	-5.845	4	0	1	0	1	0	1	0	1
343	M8	1	max	3470.355	2	0	1	0	1	0	1	0	1	1
344		min	-1139.728	3	0	1	0	1	0	1	0	1	0	1
345	2	max	3470.525	2	0	1	0	1	0	1	0	1	0	1
346		min	-1139.6	3	0	1	0	1	0	1	0	1	0	1
347	3	max	3470.696	2	0	1	0	1	0	1	0	1	0	1
348		min	-1139.472	3	0	1	0	1	0	1	0	1	0	1
349	4	max	3470.866	2	0	1	0	1	0	1	0	1	0	1
350		min	-1139.344	3	0	1	0	1	0	1	0	1	0	1
351	5	max	3471.036	2	0	1	0	1	0	1	0	1	0	1
352		min	-1139.217	3	0	1	0	1	0	1	0	1	0	1
353	6	max	3471.207	2	0	1	0	1	0	1	0	1	0	1
354		min	-1139.089	3	0	1	0	1	0	1	0	1	0	1
355	7	max	3471.377	2	0	1	0	1	0	1	0	1	0	1
356		min	-1138.961	3	0	1	0	1	0	1	0	1	0	1
357	8	max	3471.547	2	0	1	0	1	0	1	0	1	0	1
358		min	-1138.833	3	0	1	0	1	0	1	0	1	0	1
359	9	max	3471.718	2	0	1	0	1	0	1	0	1	0	1
360		min	-1138.706	3	0	1	0	1	0	1	0	1	0	1
361	10	max	3471.888	2	0	1	0	1	0	1	0	1	0	1
362		min	-1138.578	3	0	1	0	1	0	1	0	1	0	1
363	11	max	3472.058	2	0	1	0	1	0	1	0	1	0	1
364		min	-1138.45	3	0	1	0	1	0	1	0	1	0	1
365	12	max	3472.229	2	0	1	0	1	0	1	0	1	0	1
366		min	-1138.322	3	0	1	0	1	0	1	0	1	0	1
367	13	max	3472.399	2	0	1	0	1	0	1	0	1	0	1
368		min	-1138.195	3	0	1	0	1	0	1	0	1	0	1



Company : Schletter, Inc.  
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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
369		14	max	3472.569	2	0	1	0	1	0	1	0	1	0	1
370			min	-1138.067	3	0	1	0	1	0	1	0	1	0	1
371		15	max	3472.74	2	0	1	0	1	0	1	0	1	0	1
372			min	-1137.939	3	0	1	0	1	0	1	0	1	0	1
373		16	max	3472.91	2	0	1	0	1	0	1	0	1	0	1
374			min	-1137.811	3	0	1	0	1	0	1	0	1	0	1
375		17	max	3473.08	2	0	1	0	1	0	1	0	1	0	1
376			min	-1137.683	3	0	1	0	1	0	1	0	1	0	1
377		18	max	3473.251	2	0	1	0	1	0	1	0	1	0	1
378			min	-1137.556	3	0	1	0	1	0	1	0	1	0	1
379		19	max	3473.421	2	0	1	0	1	0	1	0	1	0	1
380			min	-1137.428	3	0	1	0	1	0	1	0	1	0	1
381	M10	1	max	1120.731	2	2.029	4	-.024	15	0	1	0	2	0	1
382			min	-1520.659	3	.478	15	-.651	1	0	3	0	3	0	1
383		2	max	1121.111	2	1.995	4	-.024	15	0	1	0	15	0	15
384			min	-1520.375	3	.47	15	-.651	1	0	3	0	1	0	4
385		3	max	1121.49	2	1.962	4	-.024	15	0	1	0	15	0	15
386			min	-1520.09	3	.462	15	-.651	1	0	3	0	1	-.001	4
387		4	max	1121.869	2	1.928	4	-.024	15	0	1	0	15	0	15
388			min	-1519.806	3	.454	15	-.651	1	0	3	0	1	-.002	4
389		5	max	1122.248	2	1.895	4	-.024	15	0	1	0	15	0	15
390			min	-1519.521	3	.446	15	-.651	1	0	3	0	1	-.002	4
391		6	max	1122.628	2	1.862	4	-.024	15	0	1	0	15	0	15
392			min	-1519.237	3	.438	15	-.651	1	0	3	0	1	-.002	4
393		7	max	1123.007	2	1.828	4	-.024	15	0	1	0	15	0	15
394			min	-1518.952	3	.431	15	-.651	1	0	3	0	1	-.003	4
395		8	max	1123.386	2	1.795	4	-.024	15	0	1	0	15	0	15
396			min	-1518.668	3	.423	15	-.651	1	0	3	-.001	1	-.003	4
397		9	max	1123.765	2	1.761	4	-.024	15	0	1	0	15	0	15
398			min	-1518.384	3	.415	15	-.651	1	0	3	-.001	1	-.004	4
399		10	max	1124.145	2	1.728	4	-.024	15	0	1	0	15	-.001	15
400			min	-1518.099	3	.407	15	-.651	1	0	3	-.001	1	-.004	4
401		11	max	1124.524	2	1.695	4	-.024	15	0	1	0	15	-.001	15
402			min	-1517.815	3	.398	12	-.651	1	0	3	-.002	1	-.005	4
403		12	max	1124.903	2	1.661	4	-.024	15	0	1	0	15	-.001	15
404			min	-1517.53	3	.385	12	-.651	1	0	3	-.002	1	-.005	4
405		13	max	1125.282	2	1.628	4	-.024	15	0	1	0	15	-.001	15
406			min	-1517.246	3	.372	12	-.651	1	0	3	-.002	1	-.006	4
407		14	max	1125.662	2	1.594	4	-.024	15	0	1	0	15	-.001	15
408			min	-1516.961	3	.359	12	-.651	1	0	3	-.002	1	-.006	4
409		15	max	1126.041	2	1.561	4	-.024	15	0	1	0	15	-.002	15
410			min	-1516.677	3	.346	12	-.651	1	0	3	-.002	1	-.006	4
411		16	max	1126.42	2	1.528	4	-.024	15	0	1	0	15	-.002	15
412			min	-1516.392	3	.333	12	-.651	1	0	3	-.002	1	-.007	4
413		17	max	1126.799	2	1.494	4	-.024	15	0	1	0	15	-.002	15
414			min	-1516.108	3	.32	12	-.651	1	0	3	-.003	1	-.007	4
415		18	max	1127.179	2	1.461	2	-.024	15	0	1	0	15	-.002	15
416			min	-1515.824	3	.307	12	-.651	1	0	3	-.003	1	-.008	4
417		19	max	1127.558	2	1.435	2	-.024	15	0	1	0	15	-.002	15
418			min	-1515.539	3	.294	12	-.651	1	0	3	-.003	1	-.008	4
419	M11	1	max	583.7	2	7.983	4	-.002	15	0	1	0	15	.008	4
420			min	-714.858	3	1.877	15	-.066	1	0	3	0	1	.002	15
421		2	max	583.53	2	7.213	4	-.002	15	0	1	0	15	.005	2
422			min	-714.985	3	1.696	15	-.066	1	0	3	0	1	0	12
423		3	max	583.36	2	6.443	4	-.002	15	0	1	0	15	.003	2
424			min	-715.113	3	1.515	15	-.066	1	0	3	0	1	0	3
425		4	max	583.189	2	5.673	4	-.002	15	0	1	0	15	0	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
426			min	-715.241	3	1.334	15	-.066	1	0	3	0	1	-.002	3
427		5	max	583.019	2	4.903	4	-.002	15	0	1	0	15	0	15
428			min	-715.369	3	1.153	15	-.066	1	0	3	0	1	-.003	3
429		6	max	582.849	2	4.133	4	-.002	15	0	1	0	15	-.001	15
430			min	-715.497	3	.972	15	-.066	1	0	3	0	1	-.005	4
431		7	max	582.678	2	3.363	4	-.002	15	0	1	0	15	-.001	15
432			min	-715.624	3	.791	15	-.066	1	0	3	0	1	-.006	4
433		8	max	582.508	2	2.593	4	-.002	15	0	1	0	15	-.002	15
434			min	-715.752	3	.61	15	-.066	1	0	3	0	1	-.008	4
435		9	max	582.338	2	1.823	4	-.002	15	0	1	0	15	-.002	15
436			min	-715.88	3	.429	15	-.066	1	0	3	0	1	-.009	4
437		10	max	582.167	2	1.053	4	-.002	15	0	1	0	15	-.002	15
438			min	-716.008	3	.248	15	-.066	1	0	3	0	1	-.009	4
439		11	max	581.997	2	.417	2	-.002	15	0	1	0	15	-.002	15
440			min	-716.135	3	-.101	3	-.066	1	0	3	0	1	-.009	4
441		12	max	581.827	2	-.114	15	-.002	15	0	1	0	15	-.002	15
442			min	-716.263	3	-.551	3	-.066	1	0	3	0	1	-.009	4
443		13	max	581.656	2	-.295	15	-.002	15	0	1	0	15	-.002	15
444			min	-716.391	3	-1.257	4	-.066	1	0	3	0	1	-.009	4
445		14	max	581.486	2	-.476	15	-.002	15	0	1	0	15	-.002	15
446			min	-716.519	3	-2.027	4	-.066	1	0	3	0	1	-.008	4
447		15	max	581.316	2	-.657	15	-.002	15	0	1	0	15	-.002	15
448			min	-716.646	3	-2.797	4	-.066	1	0	3	0	1	-.007	4
449		16	max	581.145	2	-.838	15	-.002	15	0	1	0	15	-.001	15
450			min	-716.774	3	-3.567	4	-.066	1	0	3	0	1	-.006	4
451		17	max	580.975	2	-1.019	15	-.002	15	0	1	0	15	-.001	15
452			min	-716.902	3	-4.337	4	-.066	1	0	3	0	1	-.004	4
453		18	max	580.805	2	-1.2	15	-.002	15	0	1	0	15	0	15
454			min	-717.03	3	-5.107	4	-.066	1	0	3	0	1	-.002	4
455		19	max	580.634	2	-1.381	15	-.002	15	0	1	0	15	0	1
456			min	-717.157	3	-5.877	4	-.066	1	0	3	0	1	0	1
457	M12	1	max	1172.358	1	0	1	6.92	1	0	1	0	15	0	1
458			min	-334.59	3	0	1	.254	15	0	1	0	1	0	1
459		2	max	1172.528	1	0	1	6.92	1	0	1	0	1	0	1
460			min	-334.462	3	0	1	.254	15	0	1	0	12	0	1
461		3	max	1172.699	1	0	1	6.92	1	0	1	.001	1	0	1
462			min	-334.334	3	0	1	.254	15	0	1	0	15	0	1
463		4	max	1172.869	1	0	1	6.92	1	0	1	.002	1	0	1
464			min	-334.206	3	0	1	.254	15	0	1	0	15	0	1
465		5	max	1173.039	1	0	1	6.92	1	0	1	.003	1	0	1
466			min	-334.079	3	0	1	.254	15	0	1	0	15	0	1
467		6	max	1173.21	1	0	1	6.92	1	0	1	.004	1	0	1
468			min	-333.951	3	0	1	.254	15	0	1	0	15	0	1
469		7	max	1173.38	1	0	1	6.92	1	0	1	.004	1	0	1
470			min	-333.823	3	0	1	.254	15	0	1	0	15	0	1
471		8	max	1173.55	1	0	1	6.92	1	0	1	.005	1	0	1
472			min	-333.695	3	0	1	.254	15	0	1	0	15	0	1
473		9	max	1173.721	1	0	1	6.92	1	0	1	.006	1	0	1
474			min	-333.568	3	0	1	.254	15	0	1	0	15	0	1
475		10	max	1173.891	1	0	1	6.92	1	0	1	.007	1	0	1
476			min	-333.44	3	0	1	.254	15	0	1	0	15	0	1
477		11	max	1174.061	1	0	1	6.92	1	0	1	.008	1	0	1
478			min	-333.312	3	0	1	.254	15	0	1	0	15	0	1
479		12	max	1174.232	1	0	1	6.92	1	0	1	.008	1	0	1
480			min	-333.184	3	0	1	.254	15	0	1	0	15	0	1
481		13	max	1174.402	1	0	1	6.92	1	0	1	.009	1	0	1
482			min	-333.057	3	0	1	.254	15	0	1	0	15	0	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
483	14	max	1174.573	1	0	1	6.92	1	0	1	.01	1	0	1
484		min	-332.929	3	0	1	.254	15	0	1	0	15	0	1
485	15	max	1174.743	1	0	1	6.92	1	0	1	.011	1	0	1
486		min	-332.801	3	0	1	.254	15	0	1	0	15	0	1
487	16	max	1174.913	1	0	1	6.92	1	0	1	.011	1	0	1
488		min	-332.673	3	0	1	.254	15	0	1	0	15	0	1
489	17	max	1175.084	1	0	1	6.92	1	0	1	.012	1	0	1
490		min	-332.546	3	0	1	.254	15	0	1	0	15	0	1
491	18	max	1175.254	1	0	1	6.92	1	0	1	.013	1	0	1
492		min	-332.418	3	0	1	.254	15	0	1	0	15	0	1
493	19	max	1175.424	1	0	1	6.92	1	0	1	.014	1	0	1
494		min	-332.29	3	0	1	.254	15	0	1	0	15	0	1
495	M1	1	max	136.138	1	842.057	3	-2.567	15	0	.165	1	0	15
496		min	4.971	15	-482.032	2	-69.472	1	0	3	.006	15	-.016	2
497	2	max	136.628	1	841.048	3	-2.567	15	0	1	.129	1	.239	2
498		min	5.119	15	-483.378	2	-69.472	1	0	3	.005	15	-.445	3
499	3	max	431.42	3	589.134	2	-2.543	15	0	3	.092	1	.481	2
500		min	-254.814	2	-624.315	3	-68.965	1	0	2	.003	15	-.87	3
501	4	max	431.787	3	587.788	2	-2.543	15	0	3	.056	1	.184	1
502		min	-254.324	2	-625.325	3	-68.965	1	0	2	.002	15	-.541	3
503	5	max	432.154	3	586.442	2	-2.543	15	0	3	.019	1	-.004	15
504		min	-253.834	2	-626.334	3	-68.965	1	0	2	0	15	-.21	3
505	6	max	432.522	3	585.096	2	-2.543	15	0	3	0	15	.12	3
506		min	-253.344	2	-627.344	3	-68.965	1	0	2	-.017	1	-.448	2
507	7	max	432.889	3	583.75	2	-2.543	15	0	3	-.002	15	.452	3
508		min	-252.854	2	-628.354	3	-68.965	1	0	2	-.054	1	-.757	2
509	8	max	433.257	3	582.404	2	-2.543	15	0	3	-.003	15	.783	3
510		min	-252.364	2	-629.363	3	-68.965	1	0	2	-.09	1	-1.064	2
511	9	max	443.066	3	53.513	2	-3.862	15	0	9	.055	1	.914	3
512		min	-197.182	2	.409	15	-104.775	1	0	3	.002	15	-1.217	2
513	10	max	443.433	3	52.167	2	-3.862	15	0	9	0	10	.891	3
514		min	-196.692	2	.003	15	-104.775	1	0	3	0	1	-1.245	2
515	11	max	443.801	3	50.821	2	-3.862	15	0	9	-.002	15	.869	3
516		min	-196.202	2	-1.668	4	-104.775	1	0	3	-.056	1	-1.273	2
517	12	max	453.478	3	414.767	3	-2.483	15	0	2	.089	1	.759	3
518		min	-140.964	2	-694.034	2	-67.525	1	0	3	.003	15	-1.128	2
519	13	max	453.845	3	413.757	3	-2.483	15	0	2	.053	1	.54	3
520		min	-140.474	2	-695.38	2	-67.525	1	0	3	.002	15	-.762	2
521	14	max	454.213	3	412.748	3	-2.483	15	0	2	.018	1	.322	3
522		min	-139.984	2	-696.727	2	-67.525	1	0	3	0	15	-.395	2
523	15	max	454.58	3	411.738	3	-2.483	15	0	2	0	15	.105	3
524		min	-139.494	2	-698.073	2	-67.525	1	0	3	-.018	1	-.049	1
525	16	max	454.947	3	410.729	3	-2.483	15	0	2	-.002	15	.342	2
526		min	-139.004	2	-699.419	2	-67.525	1	0	3	-.054	1	-.112	3
527	17	max	455.315	3	409.719	3	-2.483	15	0	2	-.003	15	.712	2
528		min	-138.514	2	-700.765	2	-67.525	1	0	3	-.089	1	-.329	3
529	18	max	-5.125	15	700.877	2	-2.734	15	0	3	-.005	15	.358	2
530		min	-136.903	1	-332.47	3	-74.168	1	0	2	-.127	1	-.162	3
531	19	max	-4.977	15	699.531	2	-2.734	15	0	3	-.006	15	.013	3
532		min	-136.413	1	-333.48	3	-74.168	1	0	2	-.167	1	-.011	1
533	M5	1	max	299.238	1	2803.463	3	0	1	0	0	1	.032	2
534		min	8.767	12	-1653.396	2	0	1	0	1	0	1	0	15
535	2	max	299.728	1	2802.453	3	0	1	0	1	0	1	.905	2
536		min	9.012	12	-1654.742	2	0	1	0	1	0	1	-1.478	3
537	3	max	1369.719	3	1723.31	2	0	1	0	1	0	1	1.738	2
538		min	-859.383	2	-1944.066	3	0	1	0	1	0	1	-2.9	3
539	4	max	1370.087	3	1721.964	2	0	1	0	1	0	1	.84	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
540			min	-858.893	2	-1945.076	3	0	1	0	1	0	1	-1.873	3
541		5	max	1370.454	3	1720.618	2	0	1	0	1	0	1	.02	9
542			min	-858.403	2	-1946.085	3	0	1	0	1	0	1	-.847	3
543		6	max	1370.822	3	1719.272	2	0	1	0	1	0	1	.18	3
544			min	-857.913	2	-1947.095	3	0	1	0	1	0	1	-.987	2
545		7	max	1371.189	3	1717.926	2	0	1	0	1	0	1	1.208	3
546			min	-857.423	2	-1948.104	3	0	1	0	1	0	1	-1.894	2
547		8	max	1371.557	3	1716.58	2	0	1	0	1	0	1	2.236	3
548			min	-856.933	2	-1949.114	3	0	1	0	1	0	1	-2.8	2
549		9	max	1384.198	3	180.022	2	0	1	0	1	0	1	2.573	3
550			min	-739.885	2	.405	15	0	1	0	1	0	1	-3.192	2
551		10	max	1384.566	3	178.676	2	0	1	0	1	0	1	2.491	3
552			min	-739.395	2	0	15	0	1	0	1	0	1	-3.286	2
553		11	max	1384.933	3	177.33	2	0	1	0	1	0	1	2.409	3
554			min	-738.905	2	-1.59	4	0	1	0	1	0	1	-3.38	2
555		12	max	1397.839	3	1263.736	3	0	1	0	1	0	1	2.114	3
556			min	-621.968	2	-2080.155	2	0	1	0	1	0	1	-3.026	2
557		13	max	1398.206	3	1262.726	3	0	1	0	1	0	1	1.447	3
558			min	-621.478	2	-2081.501	2	0	1	0	1	0	1	-1.928	2
559		14	max	1398.574	3	1261.717	3	0	1	0	1	0	1	.781	3
560			min	-620.988	2	-2082.847	2	0	1	0	1	0	1	-.83	2
561		15	max	1398.941	3	1260.707	3	0	1	0	1	0	1	.27	2
562			min	-620.498	2	-2084.193	2	0	1	0	1	0	1	-.002	13
563		16	max	1399.309	3	1259.698	3	0	1	0	1	0	1	1.37	2
564			min	-620.008	2	-2085.539	2	0	1	0	1	0	1	-.549	3
565		17	max	1399.676	3	1258.688	3	0	1	0	1	0	1	2.471	2
566			min	-619.518	2	-2086.885	2	0	1	0	1	0	1	-1.214	3
567		18	max	-9.549	12	2370.599	2	0	1	0	1	0	1	1.273	2
568			min	-299.178	1	-1152.377	3	0	1	0	1	0	1	-.635	3
569		19	max	-9.304	12	2369.253	2	0	1	0	1	0	1	.023	1
570			min	-298.688	1	-1153.387	3	0	1	0	1	0	1	-.027	3
571	M9	1	max	136.138	1	842.057	3	69.472	1	0	3	-.006	15	0	15
572			min	4.971	15	-482.032	2	2.567	15	0	1	-.165	1	-.016	2
573		2	max	136.628	1	841.048	3	69.472	1	0	3	-.005	15	.239	2
574			min	5.119	15	-483.378	2	2.567	15	0	1	-.129	1	-.445	3
575		3	max	431.42	3	589.134	2	68.965	1	0	2	-.003	15	.481	2
576			min	-254.814	2	-624.315	3	2.543	15	0	3	-.092	1	-.87	3
577		4	max	431.787	3	587.788	2	68.965	1	0	2	-.002	15	.184	1
578			min	-254.324	2	-625.325	3	2.543	15	0	3	-.056	1	-.541	3
579		5	max	432.154	3	586.442	2	68.965	1	0	2	0	15	-.004	15
580			min	-253.834	2	-626.334	3	2.543	15	0	3	-.019	1	-.21	3
581		6	max	432.522	3	585.096	2	68.965	1	0	2	.017	1	.12	3
582			min	-253.344	2	-627.344	3	2.543	15	0	3	0	15	-.448	2
583		7	max	432.889	3	583.75	2	68.965	1	0	2	.054	1	.452	3
584			min	-252.854	2	-628.354	3	2.543	15	0	3	.002	15	-.757	2
585		8	max	433.257	3	582.404	2	68.965	1	0	2	.09	1	.783	3
586			min	-252.364	2	-629.363	3	2.543	15	0	3	.003	15	-1.064	2
587		9	max	443.066	3	53.513	2	104.775	1	0	3	-.002	15	.914	3
588			min	-197.182	2	.409	15	3.862	15	0	9	-.055	1	-1.217	2
589		10	max	443.433	3	52.167	2	104.775	1	0	3	0	1	.891	3
590			min	-196.692	2	.003	15	3.862	15	0	9	0	10	-1.245	2
591		11	max	443.801	3	50.821	2	104.775	1	0	3	.056	1	.869	3
592			min	-196.202	2	-1.668	4	3.862	15	0	9	.002	15	-1.273	2
593		12	max	453.478	3	414.767	3	67.525	1	0	3	-.003	15	.759	3
594			min	-140.964	2	-694.034	2	2.483	15	0	2	-.089	1	-1.128	2
595		13	max	453.845	3	413.757	3	67.525	1	0	3	-.002	15	.54	3
596			min	-140.474	2	-695.38	2	2.483	15	0	2	-.053	1	-.762	2



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
597	14	max	454.213	3	412.748	3	67.525	1	0	3	0	15	.322	3
598		min	-139.984	2	-696.727	2	2.483	15	0	2	-.018	1	-.395	2
599	15	max	454.58	3	411.738	3	67.525	1	0	3	.018	1	.105	3
600		min	-139.494	2	-698.073	2	2.483	15	0	2	0	15	-.049	1
601	16	max	454.947	3	410.729	3	67.525	1	0	3	.054	1	.342	2
602		min	-139.004	2	-699.419	2	2.483	15	0	2	.002	15	-.112	3
603	17	max	455.315	3	409.719	3	67.525	1	0	3	.089	1	.712	2
604		min	-138.514	2	-700.765	2	2.483	15	0	2	.003	15	-.329	3
605	18	max	-5.125	15	700.877	2	74.168	1	0	2	.127	1	.358	2
606		min	-136.903	1	-332.47	3	2.734	15	0	3	.005	15	-.162	3
607	19	max	-4.977	15	699.531	2	74.168	1	0	2	.167	1	.013	3
608		min	-136.413	1	-333.48	3	2.734	15	0	3	.006	15	-.011	1

### Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M13	1	max	0	1	.132	2	.008	3	1.071e-2	2	NC	1	NC	1
2			min	0	15	-.03	3	-.004	2	-2.427e-3	3	NC	1	NC	1
3		2	max	0	1	.216	3	.02	1	1.204e-2	2	NC	4	NC	1
4			min	0	15	-.007	9	-.001	10	-2.391e-3	3	830.929	3	NC	1
5		3	max	0	1	.415	3	.047	1	1.338e-2	2	NC	5	NC	2
6			min	0	15	-.095	1	0	10	-2.354e-3	3	458.998	3	4327.056	1
7		4	max	0	1	.536	3	.071	1	1.471e-2	2	NC	5	NC	3
8			min	0	15	-.144	1	.002	10	-2.317e-3	3	360.73	3	2898.16	1
9		5	max	0	1	.564	3	.082	1	1.605e-2	2	NC	5	NC	3
10			min	0	15	-.142	1	.002	10	-2.281e-3	3	343.375	3	2493.026	1
11		6	max	0	1	.503	3	.078	1	1.739e-2	2	NC	5	NC	3
12			min	0	15	-.09	1	.001	10	-2.244e-3	3	383.289	3	2611.944	1
13		7	max	0	1	.369	3	.06	1	1.872e-2	2	NC	5	NC	2
14			min	0	15	-.011	9	-.002	10	-2.208e-3	3	511.759	3	3393.274	1
15		8	max	0	1	.199	3	.033	1	2.006e-2	2	NC	1	NC	2
16			min	0	15	.002	15	-.005	10	-2.171e-3	3	892.209	3	6172.888	1
17		9	max	0	1	.237	2	.024	3	2.139e-2	2	NC	4	NC	1
18			min	0	15	.005	15	-.011	2	-2.134e-3	3	1929.409	2	NC	1
19		10	max	0	1	.281	2	.024	3	2.273e-2	2	NC	3	NC	1
20			min	0	1	-.025	3	-.016	2	-2.098e-3	3	1366.293	2	NC	1
21		11	max	0	15	.237	2	.024	3	2.139e-2	2	NC	4	NC	1
22			min	0	1	.005	15	-.011	2	-2.134e-3	3	1929.409	2	NC	1
23		12	max	0	15	.199	3	.033	1	2.006e-2	2	NC	1	NC	2
24			min	0	1	.002	15	-.005	10	-2.171e-3	3	892.209	3	6172.888	1
25		13	max	0	15	.369	3	.06	1	1.872e-2	2	NC	5	NC	2
26			min	0	1	-.011	9	-.002	10	-2.208e-3	3	511.759	3	3393.274	1
27		14	max	0	15	.503	3	.078	1	1.739e-2	2	NC	5	NC	3
28			min	0	1	-.09	1	.001	10	-2.244e-3	3	383.289	3	2611.944	1
29		15	max	0	15	.564	3	.082	1	1.605e-2	2	NC	5	NC	3
30			min	0	1	-.142	1	.002	10	-2.281e-3	3	343.375	3	2493.026	1
31		16	max	0	15	.536	3	.071	1	1.471e-2	2	NC	5	NC	3
32			min	0	1	-.144	1	.002	10	-2.317e-3	3	360.73	3	2898.16	1
33		17	max	0	15	.415	3	.047	1	1.338e-2	2	NC	5	NC	2
34			min	0	1	-.095	1	0	10	-2.354e-3	3	458.998	3	4327.056	1
35		18	max	0	15	.216	3	.02	1	1.204e-2	2	NC	4	NC	1
36			min	0	1	-.007	9	-.001	10	-2.391e-3	3	830.929	3	NC	1
37	19	max	0	15	.132	2	.008	3	1.071e-2	2	NC	1	NC	1	
38		min	0	1	-.03	3	-.004	2	-2.427e-3	3	NC	1	NC	1	
39	M14	1	max	0	1	.274	3	.007	3	6.175e-3	2	NC	1	NC	1
40			min	0	15	-.407	2	-.004	2	-4.865e-3	3	NC	1	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
41	2	max	0	1	.538	3	.014	1	7.299e-3	2	NC	5	NC	1
42		min	0	15	-.655	2	-.002	10	-5.835e-3	3	772.021	3	NC	1
43	3	max	0	1	.765	3	.037	1	8.422e-3	2	NC	5	NC	2
44		min	0	15	-.873	2	0	10	-6.805e-3	3	415.414	3	5532.861	1
45	4	max	0	1	.928	3	.06	1	9.545e-3	2	NC	5	NC	3
46		min	0	15	-1.04	2	.002	10	-7.775e-3	3	311.574	3	3448.599	1
47	5	max	0	1	1.016	3	.072	1	1.067e-2	2	NC	15	NC	3
48		min	0	15	-1.146	2	.002	10	-8.745e-3	3	274.867	3	2850.929	1
49	6	max	0	1	1.027	3	.071	1	1.179e-2	2	NC	15	NC	3
50		min	0	15	-1.188	2	0	10	-9.716e-3	3	261.228	2	2911.418	1
51	7	max	0	1	.973	3	.055	1	1.291e-2	2	NC	15	NC	2
52		min	0	15	-1.176	2	-.002	10	-1.069e-2	3	265.47	2	3712.188	1
53	8	max	0	1	.881	3	.031	1	1.404e-2	2	NC	15	NC	2
54		min	0	15	-1.127	2	-.005	10	-1.166e-2	3	283.644	2	6637.229	1
55	9	max	0	1	.787	3	.021	3	1.516e-2	2	NC	5	NC	1
56		min	0	15	-1.068	2	-.01	2	-1.263e-2	3	308.604	2	NC	1
57	10	max	0	1	.742	3	.021	3	1.628e-2	2	NC	5	NC	1
58		min	0	1	-1.039	2	-.015	2	-1.36e-2	3	323.049	2	NC	1
59	11	max	0	15	.787	3	.021	3	1.516e-2	2	NC	5	NC	1
60		min	0	1	-1.068	2	-.01	2	-1.263e-2	3	308.604	2	NC	1
61	12	max	0	15	.881	3	.031	1	1.404e-2	2	NC	15	NC	2
62		min	0	1	-1.127	2	-.005	10	-1.166e-2	3	283.644	2	6637.229	1
63	13	max	0	15	.973	3	.055	1	1.291e-2	2	NC	15	NC	2
64		min	0	1	-1.176	2	-.002	10	-1.069e-2	3	265.47	2	3712.188	1
65	14	max	0	15	1.027	3	.071	1	1.179e-2	2	NC	15	NC	3
66		min	0	1	-1.188	2	0	10	-9.716e-3	3	261.228	2	2911.418	1
67	15	max	0	15	1.016	3	.072	1	1.067e-2	2	NC	15	NC	3
68		min	0	1	-1.146	2	.002	10	-8.745e-3	3	274.867	3	2850.929	1
69	16	max	0	15	.928	3	.06	1	9.545e-3	2	NC	5	NC	3
70		min	0	1	-1.04	2	.002	10	-7.775e-3	3	311.574	3	3448.599	1
71	17	max	0	15	.765	3	.037	1	8.422e-3	2	NC	5	NC	2
72		min	0	1	-.873	2	0	10	-6.805e-3	3	415.414	3	5532.861	1
73	18	max	0	15	.538	3	.014	1	7.299e-3	2	NC	5	NC	1
74		min	0	1	-.655	2	-.002	10	-5.835e-3	3	772.021	3	NC	1
75	19	max	0	15	.274	3	.007	3	6.175e-3	2	NC	1	NC	1
76		min	0	1	-.407	2	-.004	2	-4.865e-3	3	NC	1	NC	1
77	M15	1	max	0	15	.28	.006	3	4.14e-3	3	NC	1	NC	1
78		min	0	1	-.407	2	-.004	2	-6.4e-3	2	NC	1	NC	1
79	2	max	0	15	.458	3	.014	1	4.965e-3	3	NC	5	NC	1
80		min	0	1	-.717	2	-.001	10	-7.568e-3	2	657.811	2	NC	1
81	3	max	0	15	.616	3	.038	1	5.79e-3	3	NC	5	NC	2
82		min	0	1	-.985	2	0	10	-8.735e-3	2	352.582	2	5512.484	1
83	4	max	0	15	.738	3	.06	1	6.615e-3	3	NC	5	NC	3
84		min	0	1	-1.183	2	.002	10	-9.902e-3	2	262.692	2	3437.402	1
85	5	max	0	15	.817	3	.072	1	7.44e-3	3	NC	15	NC	3
86		min	0	1	-1.296	2	.002	10	-1.107e-2	2	229.413	2	2841.361	1
87	6	max	0	15	.851	3	.071	1	8.265e-3	3	NC	15	NC	3
88		min	0	1	-1.322	2	.001	10	-1.224e-2	2	222.786	2	2899.792	1
89	7	max	0	15	.847	3	.056	1	9.09e-3	3	NC	15	NC	2
90		min	0	1	-1.276	2	-.001	10	-1.34e-2	2	234.743	2	3691.205	1
91	8	max	0	15	.816	3	.032	1	9.915e-3	3	NC	15	NC	2
92		min	0	1	-1.183	2	-.004	10	-1.457e-2	2	262.749	2	6565.646	1
93	9	max	0	15	.777	3	.02	3	1.074e-2	3	NC	5	NC	1
94		min	0	1	-1.086	2	-.009	2	-1.574e-2	2	300.503	2	NC	1
95	10	max	0	1	.757	3	.02	3	1.157e-2	3	NC	5	NC	1
96		min	0	1	-1.038	2	-.014	2	-1.691e-2	2	323.057	2	NC	1
97	11	max	0	1	.777	3	.02	3	1.074e-2	3	NC	5	NC	1





Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
98		min	0	15	-1.086	2	-.009	2	-1.574e-2	2	300.503	2	NC	1
99		max	0	1	.816	3	.032	1	9.915e-3	3	NC	15	NC	2
100		min	0	15	-1.183	2	-.004	10	-1.457e-2	2	262.749	2	6565.646	1
101		max	0	1	.847	3	.056	1	9.09e-3	3	NC	15	NC	2
102		min	0	15	-1.276	2	-.001	10	-1.34e-2	2	234.743	2	3691.205	1
103		max	0	1	.851	3	.071	1	8.265e-3	3	NC	15	NC	3
104		min	0	15	-1.322	2	.001	10	-1.224e-2	2	222.786	2	2899.792	1
105		max	0	1	.817	3	.072	1	7.44e-3	3	NC	15	NC	3
106		min	0	15	-1.296	2	.002	10	-1.107e-2	2	229.413	2	2841.361	1
107		max	0	1	.738	3	.06	1	6.615e-3	3	NC	5	NC	3
108		min	0	15	-1.183	2	.002	10	-9.902e-3	2	262.692	2	3437.402	1
109		max	0	1	.616	3	.038	1	5.79e-3	3	NC	5	NC	2
110		min	0	15	-.985	2	0	10	-8.735e-3	2	352.582	2	5512.484	1
111		max	0	1	.458	3	.014	1	4.965e-3	3	NC	5	NC	1
112		min	0	15	-.717	2	-.001	10	-7.568e-3	2	657.811	2	NC	1
113		max	0	1	.28	3	.006	3	4.14e-3	3	NC	1	NC	1
114		min	0	15	-.407	2	-.004	2	-6.4e-3	2	NC	1	NC	1
115	M16	max	0	15	.116	2	.006	3	7.503e-3	3	NC	1	NC	1
116		min	0	1	-.094	3	-.003	2	-8.963e-3	2	NC	1	NC	1
117		max	0	15	.002	13	.02	1	8.61e-3	3	NC	4	NC	1
118		min	0	1	-.07	2	0	10	-9.897e-3	2	1097.027	2	NC	1
119		max	0	15	.043	3	.048	1	9.717e-3	3	NC	5	NC	2
120		min	0	1	-.217	2	.002	10	-1.083e-2	2	611.988	2	4327.942	1
121		max	0	15	.072	3	.071	1	1.082e-2	3	NC	5	NC	3
122		min	0	1	-.3	2	.003	15	-1.176e-2	2	489.924	2	2890.954	1
123		max	0	15	.065	3	.083	1	1.193e-2	3	NC	5	NC	3
124		min	0	1	-.307	2	.003	15	-1.27e-2	2	481.889	2	2479.797	1
125		max	0	15	.023	3	.079	1	1.304e-2	3	NC	5	NC	3
126		min	0	1	-.24	2	.003	10	-1.363e-2	2	572.426	2	2587.577	1
127		max	0	15	.001	13	.062	1	1.415e-2	3	NC	4	NC	2
128		min	0	1	-.116	2	0	10	-1.457e-2	2	880.405	2	3335.878	1
129		max	0	15	.061	1	.035	1	1.525e-2	3	NC	4	NC	2
130		min	0	1	-.124	3	-.003	10	-1.55e-2	2	2565.796	2	5940.096	1
131		max	0	15	.176	1	.018	3	1.636e-2	3	NC	4	NC	1
132		min	0	1	-.193	3	-.007	2	-1.643e-2	2	2061.174	3	NC	1
133		max	0	1	.233	2	.017	3	1.747e-2	3	NC	4	NC	1
134		min	0	1	-.224	3	-.013	2	-1.737e-2	2	1572.955	3	NC	1
135		max	0	1	.176	1	.018	3	1.636e-2	3	NC	4	NC	1
136		min	0	15	-.193	3	-.007	2	-1.643e-2	2	2061.174	3	NC	1
137		max	0	1	.061	1	.035	1	1.525e-2	3	NC	4	NC	2
138		min	0	15	-.124	3	-.003	10	-1.55e-2	2	2565.796	2	5940.096	1
139		max	0	1	.001	13	.062	1	1.415e-2	3	NC	4	NC	2
140		min	0	15	-.116	2	0	10	-1.457e-2	2	880.405	2	3335.878	1
141		max	0	1	.023	3	.079	1	1.304e-2	3	NC	5	NC	3
142		min	0	15	-.24	2	.003	10	-1.363e-2	2	572.426	2	2587.577	1
143		max	0	1	.065	3	.083	1	1.193e-2	3	NC	5	NC	3
144		min	0	15	-.307	2	.003	15	-1.27e-2	2	481.889	2	2479.797	1
145		max	0	1	.072	3	.071	1	1.082e-2	3	NC	5	NC	3
146		min	0	15	-.3	2	.003	15	-1.176e-2	2	489.924	2	2890.954	1
147		max	0	1	.043	3	.048	1	9.717e-3	3	NC	5	NC	2
148		min	0	15	-.217	2	.002	10	-1.083e-2	2	611.988	2	4327.942	1
149		max	0	1	.002	13	.02	1	8.61e-3	3	NC	4	NC	1
150		min	0	15	-.07	2	0	10	-9.897e-3	2	1097.027	2	NC	1
151		max	0	1	.116	2	.006	3	7.503e-3	3	NC	1	NC	1
152		min	0	15	-.094	3	-.003	2	-8.963e-3	2	NC	1	NC	1
153	M2	max	.006	2	.006	2	.005	1	-5.108e-6	15	NC	1	NC	1
154		min	-.008	3	-.01	3	0	15	-1.384e-4	1	9024.487	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
155		2	max	.006	2	.005	2	.005	1	-4.768e-6	15	NC	1	NC	1
156			min	-.008	3	-.01	3	0	15	-1.292e-4	1	NC	1	NC	1
157		3	max	.005	2	.005	2	.004	1	-4.428e-6	15	NC	1	NC	1
158			min	-.007	3	-.009	3	0	15	-1.2e-4	1	NC	1	NC	1
159		4	max	.005	2	.004	2	.004	1	-4.088e-6	15	NC	1	NC	1
160			min	-.007	3	-.009	3	0	15	-1.108e-4	1	NC	1	NC	1
161		5	max	.005	2	.003	2	.004	1	-3.748e-6	15	NC	1	NC	1
162			min	-.006	3	-.009	3	0	15	-1.015e-4	1	NC	1	NC	1
163		6	max	.004	2	.003	2	.003	1	-3.408e-6	15	NC	1	NC	1
164			min	-.006	3	-.008	3	0	15	-9.229e-5	1	NC	1	NC	1
165		7	max	.004	2	.002	2	.003	1	-3.068e-6	15	NC	1	NC	1
166			min	-.005	3	-.008	3	0	15	-8.306e-5	1	NC	1	NC	1
167		8	max	.004	2	.001	2	.002	1	-2.728e-6	15	NC	1	NC	1
168			min	-.005	3	-.007	3	0	15	-7.383e-5	1	NC	1	NC	1
169		9	max	.003	2	0	2	.002	1	-2.388e-6	15	NC	1	NC	1
170			min	-.004	3	-.007	3	0	15	-6.46e-5	1	NC	1	NC	1
171		10	max	.003	2	0	2	.002	1	-2.048e-6	15	NC	1	NC	1
172			min	-.004	3	-.006	3	0	15	-5.536e-5	1	NC	1	NC	1
173		11	max	.003	2	0	2	.001	1	-1.708e-6	15	NC	1	NC	1
174			min	-.004	3	-.006	3	0	15	-4.613e-5	1	NC	1	NC	1
175		12	max	.002	2	0	2	.001	1	-1.368e-6	15	NC	1	NC	1
176			min	-.003	3	-.005	3	0	15	-3.69e-5	1	NC	1	NC	1
177		13	max	.002	2	0	2	0	1	-1.028e-6	15	NC	1	NC	1
178			min	-.003	3	-.004	3	0	15	-2.767e-5	1	NC	1	NC	1
179		14	max	.002	2	0	15	0	1	-6.884e-7	15	NC	1	NC	1
180			min	-.002	3	-.004	3	0	15	-1.844e-5	1	NC	1	NC	1
181		15	max	.001	2	0	15	0	1	-3.484e-7	15	NC	1	NC	1
182			min	-.002	3	-.003	3	0	15	-9.208e-6	1	NC	1	NC	1
183		16	max	0	2	0	15	0	1	3.362e-7	2	NC	1	NC	1
184			min	-.001	3	-.002	3	0	15	-7.796e-7	3	NC	1	NC	1
185		17	max	0	2	0	15	0	1	9.255e-6	1	NC	1	NC	1
186			min	0	3	-.002	3	0	15	1.141e-7	12	NC	1	NC	1
187		18	max	0	2	0	15	0	1	1.849e-5	1	NC	1	NC	1
188			min	0	3	0	3	0	15	6.715e-7	15	NC	1	NC	1
189		19	max	0	1	0	1	0	1	2.772e-5	1	NC	1	NC	1
190			min	0	1	0	1	0	1	1.012e-6	15	NC	1	NC	1
191	M3	1	max	0	1	0	1	0	1	-3.225e-7	15	NC	1	NC	1
192			min	0	1	0	1	0	1	-8.824e-6	1	NC	1	NC	1
193		2	max	0	3	0	15	0	1	7.504e-6	1	NC	1	NC	1
194			min	0	2	-.002	4	0	15	2.764e-7	15	NC	1	NC	1
195		3	max	0	3	0	15	0	1	2.383e-5	1	NC	1	NC	1
196			min	0	2	-.003	4	0	15	8.753e-7	15	NC	1	NC	1
197		4	max	.001	3	-.001	15	0	1	4.016e-5	1	NC	1	NC	1
198			min	0	2	-.005	4	0	15	1.474e-6	15	NC	1	NC	1
199		5	max	.001	3	-.002	15	0	1	5.649e-5	1	NC	1	NC	1
200			min	-.001	2	-.007	4	0	15	2.073e-6	15	NC	1	NC	1
201		6	max	.002	3	-.002	15	0	1	7.282e-5	1	NC	1	NC	1
202			min	-.001	2	-.009	4	0	15	2.672e-6	15	NC	1	NC	1
203		7	max	.002	3	-.002	15	.001	1	8.914e-5	1	NC	1	NC	1
204			min	-.002	2	-.01	4	0	15	3.271e-6	15	8934.464	4	NC	1
205		8	max	.002	3	-.003	15	.001	1	1.055e-4	1	NC	1	NC	1
206			min	-.002	2	-.012	4	0	15	3.87e-6	15	7993.303	4	NC	1
207		9	max	.003	3	-.003	15	.001	1	1.218e-4	1	NC	1	NC	1
208			min	-.002	2	-.013	4	0	15	4.469e-6	15	7433.686	4	NC	1
209		10	max	.003	3	-.003	15	.002	1	1.381e-4	1	NC	2	NC	1
210			min	-.003	2	-.013	4	0	15	5.068e-6	15	7157.235	4	NC	1
211		11	max	.003	3	-.003	15	.002	1	1.545e-4	1	NC	2	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
212		min	-.003	2	-.013	4	0	15	5.667e-6	15	7122.904	4	NC	1
213		max	.004	3	-.003	15	.002	1	1.708e-4	1	NC	2	NC	1
214		min	-.003	2	-.013	4	0	15	6.265e-6	15	7331.199	4	NC	1
215		max	.004	3	-.003	15	.003	1	1.871e-4	1	NC	1	NC	1
216		min	-.003	2	-.012	4	0	15	6.864e-6	15	7826.347	4	NC	1
217		max	.005	3	-.003	15	.003	1	2.034e-4	1	NC	1	NC	1
218		min	-.004	2	-.011	4	0	15	7.463e-6	15	8719.541	4	NC	1
219		max	.005	3	-.002	15	.003	1	2.198e-4	1	NC	1	NC	1
220		min	-.004	2	-.009	4	0	15	8.062e-6	15	NC	1	NC	1
221		max	.005	3	-.002	15	.004	1	2.361e-4	1	NC	1	NC	1
222		min	-.004	2	-.008	4	0	15	8.661e-6	15	NC	1	NC	1
223		max	.006	3	-.001	15	.004	1	2.524e-4	1	NC	1	NC	1
224		min	-.005	2	-.006	1	0	15	9.26e-6	15	NC	1	NC	1
225		max	.006	3	0	15	.005	1	2.688e-4	1	NC	1	NC	1
226		min	-.005	2	-.004	1	0	15	9.859e-6	15	NC	1	NC	1
227		max	.006	3	0	15	.005	1	2.851e-4	1	NC	1	NC	1
228		min	-.005	2	-.003	1	0	15	1.046e-5	15	NC	1	NC	1
229	M4	max	.003	1	.005	2	0	15	1.924e-5	1	NC	1	NC	2
230		min	0	3	-.006	3	-.005	1	7.17e-7	15	NC	1	4802.781	1
231		max	.003	1	.004	2	0	15	1.924e-5	1	NC	1	NC	2
232		min	0	3	-.006	3	-.005	1	7.17e-7	15	NC	1	5226.424	1
233		max	.002	1	.004	2	0	15	1.924e-5	1	NC	1	NC	2
234		min	0	3	-.006	3	-.004	1	7.17e-7	15	NC	1	5730.413	1
235		max	.002	1	.004	2	0	15	1.924e-5	1	NC	1	NC	2
236		min	0	3	-.005	3	-.004	1	7.17e-7	15	NC	1	6335.655	1
237		max	.002	1	.004	2	0	15	1.924e-5	1	NC	1	NC	2
238		min	0	3	-.005	3	-.004	1	7.17e-7	15	NC	1	7070.567	1
239		max	.002	1	.003	2	0	15	1.924e-5	1	NC	1	NC	2
240		min	0	3	-.005	3	-.003	1	7.17e-7	15	NC	1	7974.553	1
241		max	.002	1	.003	2	0	15	1.924e-5	1	NC	1	NC	2
242		min	0	3	-.004	3	-.003	1	7.17e-7	15	NC	1	9103.551	1
243		max	.002	1	.003	2	0	15	1.924e-5	1	NC	1	NC	1
244		min	0	3	-.004	3	-.002	1	7.17e-7	15	NC	1	NC	1
245		max	.002	1	.003	2	0	15	1.924e-5	1	NC	1	NC	1
246		min	0	3	-.003	3	-.002	1	7.17e-7	15	NC	1	NC	1
247		max	.001	1	.002	2	0	15	1.924e-5	1	NC	1	NC	1
248		min	0	3	-.003	3	-.002	1	7.17e-7	15	NC	1	NC	1
249		max	.001	1	.002	2	0	15	1.924e-5	1	NC	1	NC	1
250		min	0	3	-.003	3	-.001	1	7.17e-7	15	NC	1	NC	1
251		max	.001	1	.002	2	0	15	1.924e-5	1	NC	1	NC	1
252		min	0	3	-.002	3	-.001	1	7.17e-7	15	NC	1	NC	1
253		max	0	1	.002	2	0	15	1.924e-5	1	NC	1	NC	1
254		min	0	3	-.002	3	0	1	7.17e-7	15	NC	1	NC	1
255		max	0	1	.001	2	0	15	1.924e-5	1	NC	1	NC	1
256		min	0	3	-.002	3	0	1	7.17e-7	15	NC	1	NC	1
257		max	0	1	.001	2	0	15	1.924e-5	1	NC	1	NC	1
258		min	0	3	-.001	3	0	1	7.17e-7	15	NC	1	NC	1
259		max	0	1	0	2	0	15	1.924e-5	1	NC	1	NC	1
260		min	0	3	-.001	3	0	1	7.17e-7	15	NC	1	NC	1
261		max	0	1	0	2	0	15	1.924e-5	1	NC	1	NC	1
262		min	0	3	0	3	0	1	7.17e-7	15	NC	1	NC	1
263		max	0	1	0	2	0	15	1.924e-5	1	NC	1	NC	1
264		min	0	3	0	3	0	1	7.17e-7	15	NC	1	NC	1
265		max	0	1	0	1	0	1	1.924e-5	1	NC	1	NC	1
266		min	0	1	0	1	0	1	7.17e-7	15	NC	1	NC	1
267	M6	max	.019	2	.022	2	0	1	0	1	NC	4	NC	1
268		min	-.026	3	-.032	3	0	1	0	1	1712.415	3	NC	1



Company : Schletter, Inc.  
Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
269	2	max	.018	2	.02	2	0	1	0	1	NC	4	NC	1
270		min	-.025	3	-.03	3	0	1	0	1	1817.093	3	NC	1
271	3	max	.017	2	.019	2	0	1	0	1	NC	4	NC	1
272		min	-.023	3	-.029	3	0	1	0	1	1935.341	3	NC	1
273	4	max	.016	2	.017	2	0	1	0	1	NC	4	NC	1
274		min	-.022	3	-.027	3	0	1	0	1	2069.904	3	NC	1
275	5	max	.015	2	.015	2	0	1	0	1	NC	4	NC	1
276		min	-.02	3	-.025	3	0	1	0	1	2224.314	3	NC	1
277	6	max	.014	2	.013	2	0	1	0	1	NC	4	NC	1
278		min	-.019	3	-.023	3	0	1	0	1	2403.191	3	NC	1
279	7	max	.013	2	.012	2	0	1	0	1	NC	1	NC	1
280		min	-.017	3	-.021	3	0	1	0	1	2612.699	3	NC	1
281	8	max	.012	2	.01	2	0	1	0	1	NC	1	NC	1
282		min	-.016	3	-.019	3	0	1	0	1	2861.242	3	NC	1
283	9	max	.011	2	.009	2	0	1	0	1	NC	1	NC	1
284		min	-.015	3	-.018	3	0	1	0	1	3160.594	3	NC	1
285	10	max	.009	2	.007	2	0	1	0	1	NC	1	NC	1
286		min	-.013	3	-.016	3	0	1	0	1	3527.761	3	NC	1
287	11	max	.008	2	.006	2	0	1	0	1	NC	1	NC	1
288		min	-.012	3	-.014	3	0	1	0	1	3988.26	3	NC	1
289	12	max	.007	2	.005	2	0	1	0	1	NC	1	NC	1
290		min	-.01	3	-.012	3	0	1	0	1	4582.198	3	NC	1
291	13	max	.006	2	.003	2	0	1	0	1	NC	1	NC	1
292		min	-.009	3	-.01	3	0	1	0	1	5376.423	3	NC	1
293	14	max	.005	2	.003	2	0	1	0	1	NC	1	NC	1
294		min	-.007	3	-.009	3	0	1	0	1	6491.277	3	NC	1
295	15	max	.004	2	.002	2	0	1	0	1	NC	1	NC	1
296		min	-.006	3	-.007	3	0	1	0	1	8167.458	3	NC	1
297	16	max	.003	2	.001	2	0	1	0	1	NC	1	NC	1
298		min	-.004	3	-.005	3	0	1	0	1	NC	1	NC	1
299	17	max	.002	2	0	2	0	1	0	1	NC	1	NC	1
300		min	-.003	3	-.003	3	0	1	0	1	NC	1	NC	1
301	18	max	.001	2	0	2	0	1	0	1	NC	1	NC	1
302		min	-.001	3	-.002	3	0	1	0	1	NC	1	NC	1
303	19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
304		min	0	1	0	1	0	1	0	1	NC	1	NC	1
305	M7	1	max	0	1	0	1	0	1	1	NC	1	NC	1
306		min	0	1	0	1	0	1	0	1	NC	1	NC	1
307	2	max	.001	3	0	2	0	1	0	1	NC	1	NC	1
308		min	-.001	2	-.003	3	0	1	0	1	NC	1	NC	1
309	3	max	.002	3	0	2	0	1	0	1	NC	1	NC	1
310		min	-.002	2	-.005	3	0	1	0	1	NC	1	NC	1
311	4	max	.003	3	-.001	15	0	1	0	1	NC	1	NC	1
312		min	-.003	2	-.007	3	0	1	0	1	NC	1	NC	1
313	5	max	.004	3	-.002	15	0	1	0	1	NC	1	NC	1
314		min	-.004	2	-.009	3	0	1	0	1	NC	1	NC	1
315	6	max	.005	3	-.002	15	0	1	0	1	NC	1	NC	1
316		min	-.005	2	-.011	3	0	1	0	1	8689.371	3	NC	1
317	7	max	.006	3	-.002	15	0	1	0	1	NC	1	NC	1
318		min	-.006	2	-.013	3	0	1	0	1	7759.186	3	NC	1
319	8	max	.008	3	-.003	15	0	1	0	1	NC	1	NC	1
320		min	-.007	2	-.014	3	0	1	0	1	7208.44	3	NC	1
321	9	max	.009	3	-.003	15	0	1	0	1	NC	1	NC	1
322		min	-.008	2	-.014	3	0	1	0	1	6923.482	3	NC	1
323	10	max	.01	3	-.003	15	0	1	0	1	NC	1	NC	1
324		min	-.009	2	-.015	3	0	1	0	1	6853.291	3	NC	1
325	11	max	.011	3	-.003	15	0	1	0	1	NC	1	NC	1



Company : Schletter, Inc.  
 Designer : HCV  
 Job Number :  
 Model Name : Standard PVMax Racking System

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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
326			min	-.01	2	-.015	3	0	1	0	1	6984.956	3	NC	1
327		12	max	.012	3	-.003	15	0	1	0	1	NC	1	NC	1
328			min	-.011	2	-.014	3	0	1	0	1	7337.874	3	NC	1
329		13	max	.013	3	-.003	15	0	1	0	1	NC	1	NC	1
330			min	-.012	2	-.013	3	0	1	0	1	7943.312	4	NC	1
331		14	max	.014	3	-.003	15	0	1	0	1	NC	1	NC	1
332			min	-.013	2	-.012	3	0	1	0	1	8845.083	4	NC	1
333		15	max	.015	3	-.002	15	0	1	0	1	NC	1	NC	1
334			min	-.014	2	-.011	3	0	1	0	1	NC	1	NC	1
335		16	max	.016	3	-.002	15	0	1	0	1	NC	1	NC	1
336			min	-.015	2	-.009	3	0	1	0	1	NC	1	NC	1
337		17	max	.017	3	-.001	15	0	1	0	1	NC	1	NC	1
338			min	-.016	2	-.008	1	0	1	0	1	NC	1	NC	1
339		18	max	.018	3	0	15	0	1	0	1	NC	1	NC	1
340			min	-.017	2	-.007	1	0	1	0	1	NC	1	NC	1
341		19	max	.019	3	0	15	0	1	0	1	NC	1	NC	1
342			min	-.018	2	-.006	1	0	1	0	1	NC	1	NC	1
343	M8	1	max	.008	2	.017	2	0	1	0	1	NC	1	NC	1
344			min	-.003	3	-.019	3	0	1	0	1	NC	1	NC	1
345		2	max	.008	2	.016	2	0	1	0	1	NC	1	NC	1
346			min	-.003	3	-.018	3	0	1	0	1	NC	1	NC	1
347		3	max	.007	2	.015	2	0	1	0	1	NC	1	NC	1
348			min	-.002	3	-.017	3	0	1	0	1	NC	1	NC	1
349		4	max	.007	2	.014	2	0	1	0	1	NC	1	NC	1
350			min	-.002	3	-.016	3	0	1	0	1	NC	1	NC	1
351		5	max	.006	2	.013	2	0	1	0	1	NC	1	NC	1
352			min	-.002	3	-.015	3	0	1	0	1	NC	1	NC	1
353		6	max	.006	2	.012	2	0	1	0	1	NC	1	NC	1
354			min	-.002	3	-.014	3	0	1	0	1	NC	1	NC	1
355		7	max	.006	2	.011	2	0	1	0	1	NC	1	NC	1
356			min	-.002	3	-.013	3	0	1	0	1	NC	1	NC	1
357		8	max	.005	2	.01	2	0	1	0	1	NC	1	NC	1
358			min	-.002	3	-.012	3	0	1	0	1	NC	1	NC	1
359		9	max	.005	2	.009	2	0	1	0	1	NC	1	NC	1
360			min	-.002	3	-.011	3	0	1	0	1	NC	1	NC	1
361		10	max	.004	2	.008	2	0	1	0	1	NC	1	NC	1
362			min	-.001	3	-.01	3	0	1	0	1	NC	1	NC	1
363		11	max	.004	2	.007	2	0	1	0	1	NC	1	NC	1
364			min	-.001	3	-.009	3	0	1	0	1	NC	1	NC	1
365		12	max	.003	2	.006	2	0	1	0	1	NC	1	NC	1
366			min	-.001	3	-.008	3	0	1	0	1	NC	1	NC	1
367		13	max	.003	2	.006	2	0	1	0	1	NC	1	NC	1
368			min	0	3	-.006	3	0	1	0	1	NC	1	NC	1
369		14	max	.002	2	.005	2	0	1	0	1	NC	1	NC	1
370			min	0	3	-.005	3	0	1	0	1	NC	1	NC	1
371		15	max	.002	2	.004	2	0	1	0	1	NC	1	NC	1
372			min	0	3	-.004	3	0	1	0	1	NC	1	NC	1
373		16	max	.001	2	.003	2	0	1	0	1	NC	1	NC	1
374			min	0	3	-.003	3	0	1	0	1	NC	1	NC	1
375		17	max	0	2	.002	2	0	1	0	1	NC	1	NC	1
376			min	0	3	-.002	3	0	1	0	1	NC	1	NC	1
377		18	max	0	2	0	2	0	1	0	1	NC	1	NC	1
378			min	0	3	-.001	3	0	1	0	1	NC	1	NC	1
379		19	max	0	1	0	1	0	1	0	1	NC	1	NC	1
380			min	0	1	0	1	0	1	0	1	NC	1	NC	1
381	M10	1	max	.006	2	.006	2	0	15	1.384e-4	1	NC	1	NC	1
382			min	-.008	3	-.01	3	-.005	1	5.108e-6	15	9024.487	2	NC	1



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Designer : HCV  
Job Number :  
Model Name : Standard PVMax Racking System

Oct 26, 2015

Checked By: \_\_\_\_\_

### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n)	L/y Ratio	LC	(n)	L/z Ratio	LC
383		2	max	.006	2	.005	2	0	15	1.292e-4	1		NC	1		NC	1
384			min	-.008	3	-.01	3	-.005	1	4.768e-6	15		NC	1		NC	1
385		3	max	.005	2	.005	2	0	15	1.2e-4	1		NC	1		NC	1
386			min	-.007	3	-.009	3	-.004	1	4.428e-6	15		NC	1		NC	1
387		4	max	.005	2	.004	2	0	15	1.108e-4	1		NC	1		NC	1
388			min	-.007	3	-.009	3	-.004	1	4.088e-6	15		NC	1		NC	1
389		5	max	.005	2	.003	2	0	15	1.015e-4	1		NC	1		NC	1
390			min	-.006	3	-.009	3	-.004	1	3.748e-6	15		NC	1		NC	1
391		6	max	.004	2	.003	2	0	15	9.229e-5	1		NC	1		NC	1
392			min	-.006	3	-.008	3	-.003	1	3.408e-6	15		NC	1		NC	1
393		7	max	.004	2	.002	2	0	15	8.306e-5	1		NC	1		NC	1
394			min	-.005	3	-.008	3	-.003	1	3.068e-6	15		NC	1		NC	1
395		8	max	.004	2	.001	2	0	15	7.383e-5	1		NC	1		NC	1
396			min	-.005	3	-.007	3	-.002	1	2.728e-6	15		NC	1		NC	1
397		9	max	.003	2	0	2	0	15	6.46e-5	1		NC	1		NC	1
398			min	-.004	3	-.007	3	-.002	1	2.388e-6	15		NC	1		NC	1
399		10	max	.003	2	0	2	0	15	5.536e-5	1		NC	1		NC	1
400			min	-.004	3	-.006	3	-.002	1	2.048e-6	15		NC	1		NC	1
401		11	max	.003	2	0	2	0	15	4.613e-5	1		NC	1		NC	1
402			min	-.004	3	-.006	3	-.001	1	1.708e-6	15		NC	1		NC	1
403		12	max	.002	2	0	2	0	15	3.69e-5	1		NC	1		NC	1
404			min	-.003	3	-.005	3	-.001	1	1.368e-6	15		NC	1		NC	1
405		13	max	.002	2	0	2	0	15	2.767e-5	1		NC	1		NC	1
406			min	-.003	3	-.004	3	0	1	1.028e-6	15		NC	1		NC	1
407		14	max	.002	2	0	15	0	15	1.844e-5	1		NC	1		NC	1
408			min	-.002	3	-.004	3	0	1	6.884e-7	15		NC	1		NC	1
409		15	max	.001	2	0	15	0	15	9.208e-6	1		NC	1		NC	1
410			min	-.002	3	-.003	3	0	1	3.484e-7	15		NC	1		NC	1
411		16	max	0	2	0	15	0	15	7.796e-7	3		NC	1		NC	1
412			min	-.001	3	-.002	3	0	1	-3.362e-7	2		NC	1		NC	1
413		17	max	0	2	0	15	0	15	-1.141e-7	12		NC	1		NC	1
414			min	0	3	-.002	3	0	1	-9.255e-6	1		NC	1		NC	1
415		18	max	0	2	0	15	0	15	-6.715e-7	15		NC	1		NC	1
416			min	0	3	0	3	0	1	-1.849e-5	1		NC	1		NC	1
417		19	max	0	1	0	1	0	1	-1.012e-6	15		NC	1		NC	1
418			min	0	1	0	1	0	1	-2.772e-5	1		NC	1		NC	1
419	M11	1	max	0	1	0	1	0	1	8.824e-6	1		NC	1		NC	1
420			min	0	1	0	1	0	1	3.225e-7	15		NC	1		NC	1
421		2	max	0	3	0	15	0	15	-2.764e-7	15		NC	1		NC	1
422			min	0	2	-.002	4	0	1	-7.504e-6	1		NC	1		NC	1
423		3	max	0	3	0	15	0	15	-8.753e-7	15		NC	1		NC	1
424			min	0	2	-.003	4	0	1	-2.383e-5	1		NC	1		NC	1
425		4	max	.001	3	-.001	15	0	15	-1.474e-6	15		NC	1		NC	1
426			min	0	2	-.005	4	0	1	-4.016e-5	1		NC	1		NC	1
427		5	max	.001	3	-.002	15	0	15	-2.073e-6	15		NC	1		NC	1
428			min	-.001	2	-.007	4	0	1	-5.649e-5	1		NC	1		NC	1
429		6	max	.002	3	-.002	15	0	15	-2.672e-6	15		NC	1		NC	1
430			min	-.001	2	-.009	4	0	1	-7.282e-5	1		NC	1		NC	1
431		7	max	.002	3	-.002	15	0	15	-3.271e-6	15		NC	1		NC	1
432			min	-.002	2	-.01	4	-.001	1	-8.914e-5	1	8934.464		4		NC	1
433		8	max	.002	3	-.003	15	0	15	-3.87e-6	15		NC	1		NC	1
434			min	-.002	2	-.012	4	-.001	1	-1.055e-4	1	7993.303		4		NC	1
435		9	max	.003	3	-.003	15	0	15	-4.469e-6	15		NC	1		NC	1
436			min	-.002	2	-.013	4	-.001	1	-1.218e-4	1	7433.686		4		NC	1
437		10	max	.003	3	-.003	15	0	15	-5.068e-6	15		NC	2		NC	1
438			min	-.003	2	-.013	4	-.002	1	-1.381e-4	1	7157.235		4		NC	1
439		11	max	.003	3	-.003	15	0	15	-5.667e-6	15		NC	2		NC	1





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### Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
440		min	-.003	2	-.013	4	-.002	1	-1.545e-4	1	7122.904	4	NC	1
441		max	.004	3	-.003	15	0	15	-6.265e-6	15	NC	2	NC	1
442		min	-.003	2	-.013	4	-.002	1	-1.708e-4	1	7331.199	4	NC	1
443		max	.004	3	-.003	15	0	15	-6.864e-6	15	NC	1	NC	1
444		min	-.003	2	-.012	4	-.003	1	-1.871e-4	1	7826.347	4	NC	1
445		max	.005	3	-.003	15	0	15	-7.463e-6	15	NC	1	NC	1
446		min	-.004	2	-.011	4	-.003	1	-2.034e-4	1	8719.541	4	NC	1
447		max	.005	3	-.002	15	0	15	-8.062e-6	15	NC	1	NC	1
448		min	-.004	2	-.009	4	-.003	1	-2.198e-4	1	NC	1	NC	1
449		max	.005	3	-.002	15	0	15	-8.661e-6	15	NC	1	NC	1
450		min	-.004	2	-.008	4	-.004	1	-2.361e-4	1	NC	1	NC	1
451		max	.006	3	-.001	15	0	15	-9.26e-6	15	NC	1	NC	1
452		min	-.005	2	-.006	1	-.004	1	-2.524e-4	1	NC	1	NC	1
453		max	.006	3	0	15	0	15	-9.859e-6	15	NC	1	NC	1
454		min	-.005	2	-.004	1	-.005	1	-2.688e-4	1	NC	1	NC	1
455		max	.006	3	0	15	0	15	-1.046e-5	15	NC	1	NC	1
456		min	-.005	2	-.003	1	-.005	1	-2.851e-4	1	NC	1	NC	1
457	M12	max	.003	1	.005	2	.005	1	-7.17e-7	15	NC	1	NC	2
458		min	0	3	-.006	3	0	15	-1.924e-5	1	NC	1	4802.781	1
459		max	.003	1	.004	2	.005	1	-7.17e-7	15	NC	1	NC	2
460		min	0	3	-.006	3	0	15	-1.924e-5	1	NC	1	5226.424	1
461		max	.002	1	.004	2	.004	1	-7.17e-7	15	NC	1	NC	2
462		min	0	3	-.006	3	0	15	-1.924e-5	1	NC	1	5730.413	1
463		max	.002	1	.004	2	.004	1	-7.17e-7	15	NC	1	NC	2
464		min	0	3	-.005	3	0	15	-1.924e-5	1	NC	1	6335.655	1
465		max	.002	1	.004	2	.004	1	-7.17e-7	15	NC	1	NC	2
466		min	0	3	-.005	3	0	15	-1.924e-5	1	NC	1	7070.567	1
467		max	.002	1	.003	2	.003	1	-7.17e-7	15	NC	1	NC	2
468		min	0	3	-.005	3	0	15	-1.924e-5	1	NC	1	7974.553	1
469		max	.002	1	.003	2	.003	1	-7.17e-7	15	NC	1	NC	2
470		min	0	3	-.004	3	0	15	-1.924e-5	1	NC	1	9103.551	1
471		max	.002	1	.003	2	.002	1	-7.17e-7	15	NC	1	NC	1
472		min	0	3	-.004	3	0	15	-1.924e-5	1	NC	1	NC	1
473		max	.002	1	.003	2	.002	1	-7.17e-7	15	NC	1	NC	1
474		min	0	3	-.003	3	0	15	-1.924e-5	1	NC	1	NC	1
475		max	.001	1	.002	2	.002	1	-7.17e-7	15	NC	1	NC	1
476		min	0	3	-.003	3	0	15	-1.924e-5	1	NC	1	NC	1
477		max	.001	1	.002	2	.001	1	-7.17e-7	15	NC	1	NC	1
478		min	0	3	-.003	3	0	15	-1.924e-5	1	NC	1	NC	1
479		max	.001	1	.002	2	.001	1	-7.17e-7	15	NC	1	NC	1
480		min	0	3	-.002	3	0	15	-1.924e-5	1	NC	1	NC	1
481		max	0	1	.002	2	0	1	-7.17e-7	15	NC	1	NC	1
482		min	0	3	-.002	3	0	15	-1.924e-5	1	NC	1	NC	1
483		max	0	1	.001	2	0	1	-7.17e-7	15	NC	1	NC	1
484		min	0	3	-.002	3	0	15	-1.924e-5	1	NC	1	NC	1
485		max	0	1	.001	2	0	1	-7.17e-7	15	NC	1	NC	1
486		min	0	3	-.001	3	0	15	-1.924e-5	1	NC	1	NC	1
487		max	0	1	0	2	0	1	-7.17e-7	15	NC	1	NC	1
488		min	0	3	-.001	3	0	15	-1.924e-5	1	NC	1	NC	1
489		max	0	1	0	2	0	1	-7.17e-7	15	NC	1	NC	1
490		min	0	3	0	3	0	15	-1.924e-5	1	NC	1	NC	1
491		max	0	1	0	2	0	1	-7.17e-7	15	NC	1	NC	1
492		min	0	3	0	3	0	15	-1.924e-5	1	NC	1	NC	1
493		max	0	1	0	1	0	1	-7.17e-7	15	NC	1	NC	1
494		min	0	1	0	1	0	1	-1.924e-5	1	NC	1	NC	1
495	M1	max	.008	3	.132	2	0	1	1.095e-2	1	NC	1	NC	1
496		min	-.004	2	-.03	3	0	15	-2.249e-2	3	NC	1	NC	1



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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
497		2	max	.008	3	.064	2	0	15	5.357e-3	2	NC	4	NC	1
498			min	-.004	2	-.014	3	-.004	1	-1.112e-2	3	1696.353	2	NC	1
499		3	max	.008	3	.011	3	0	15	3.496e-5	10	NC	5	NC	1
500			min	-.004	2	-.009	2	-.005	1	-1.191e-4	3	818.966	2	NC	1
501		4	max	.008	3	.053	3	0	15	4.167e-3	2	NC	5	NC	1
502			min	-.004	2	-.091	2	-.005	1	-4.536e-3	3	518.254	2	NC	1
503		5	max	.007	3	.105	3	0	15	8.306e-3	2	NC	5	NC	1
504			min	-.004	2	-.176	2	-.003	1	-8.953e-3	3	374.814	2	NC	1
505		6	max	.007	3	.161	3	0	15	1.244e-2	2	NC	15	NC	1
506			min	-.004	2	-.258	2	-.001	1	-1.337e-2	3	295.678	2	NC	1
507		7	max	.007	3	.215	3	0	1	1.658e-2	2	NC	15	NC	1
508			min	-.004	2	-.331	2	0	3	-1.779e-2	3	248.907	2	NC	1
509		8	max	.007	3	.26	3	0	1	2.072e-2	2	9703.718	15	NC	1
510			min	-.004	2	-.39	2	0	15	-2.22e-2	3	221.216	2	NC	1
511		9	max	.007	3	.289	3	0	15	2.345e-2	2	9077.337	15	NC	1
512			min	-.004	2	-.426	2	0	1	-2.249e-2	3	206.791	2	NC	1
513		10	max	.007	3	.299	3	0	1	2.523e-2	2	8886.238	15	NC	1
514			min	-.004	2	-.439	2	0	15	-2.004e-2	3	202.56	2	NC	1
515		11	max	.007	3	.292	3	0	1	2.701e-2	2	9077.01	15	NC	1
516			min	-.004	2	-.426	2	0	15	-1.759e-2	3	207.489	2	NC	1
517		12	max	.006	3	.268	3	0	15	2.603e-2	2	9703.016	15	NC	1
518			min	-.004	2	-.388	2	0	1	-1.492e-2	3	223.323	2	NC	1
519		13	max	.006	3	.228	3	0	15	2.087e-2	2	NC	15	NC	1
520			min	-.004	2	-.328	2	0	1	-1.195e-2	3	253.999	2	NC	1
521		14	max	.006	3	.177	3	.001	1	1.571e-2	2	NC	15	NC	1
522			min	-.003	2	-.252	2	0	15	-8.971e-3	3	306.495	2	NC	1
523		15	max	.006	3	.12	3	.003	1	1.055e-2	2	NC	5	NC	1
524			min	-.003	2	-.168	2	0	15	-5.996e-3	3	396.946	2	NC	1
525		16	max	.006	3	.061	3	.005	1	5.387e-3	2	NC	5	NC	1
526			min	-.003	2	-.083	2	0	15	-3.02e-3	3	564.618	2	NC	1
527		17	max	.006	3	.004	3	.005	1	3.902e-4	1	NC	5	NC	1
528			min	-.003	2	-.006	2	0	15	-4.359e-5	3	923.363	2	NC	1
529		18	max	.006	3	.059	2	.004	1	8.504e-3	2	NC	4	NC	1
530			min	-.003	2	-.047	3	0	15	-3.541e-3	3	1960.948	2	NC	1
531		19	max	.006	3	.116	2	0	15	1.709e-2	2	NC	1	NC	1
532			min	-.003	2	-.094	3	0	1	-7.189e-3	3	NC	1	NC	1
533	M5	1	max	.024	3	.281	2	0	1	0	1	NC	1	NC	1
534			min	-.016	2	-.025	3	0	1	0	1	NC	1	NC	1
535		2	max	.024	3	.135	2	0	1	0	1	NC	5	NC	1
536			min	-.016	2	-.01	3	0	1	0	1	797.105	2	NC	1
537		3	max	.024	3	.036	3	0	1	0	1	NC	5	NC	1
538			min	-.016	2	-.028	2	0	1	0	1	374.759	2	NC	1
539		4	max	.023	3	.135	3	0	1	0	1	NC	15	NC	1
540			min	-.016	2	-.224	2	0	1	0	1	229.168	2	NC	1
541		5	max	.023	3	.272	3	0	1	0	1	8075.215	15	NC	1
542			min	-.016	2	-.437	2	0	1	0	1	161.181	2	NC	1
543		6	max	.022	3	.426	3	0	1	0	1	6209.25	15	NC	1
544			min	-.015	2	-.648	2	0	1	0	1	124.52	2	NC	1
545		7	max	.022	3	.577	3	0	1	0	1	5133.237	15	NC	1
546			min	-.015	2	-.839	2	0	1	0	1	103.257	2	NC	1
547		8	max	.022	3	.703	3	0	1	0	1	4508.277	15	NC	1
548			min	-.015	2	-.992	2	0	1	0	1	90.857	2	NC	1
549		9	max	.021	3	.784	3	0	1	0	1	4188.044	15	NC	1
550			min	-.014	2	-1.089	2	0	1	0	1	84.485	2	NC	1
551		10	max	.021	3	.813	3	0	1	0	1	4091.601	15	NC	1
552			min	-.014	2	-1.123	2	0	1	0	1	82.621	2	NC	1
553		11	max	.02	3	.793	3	0	1	0	1	4188.184	15	NC	1





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### Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
554			min	-.014	2	-1.09	2	0	1	0	1	84.785	2	NC	1
555		12	max	.02	3	.724	3	0	1	0	1	4508.601	15	NC	1
556			min	-.014	2	-.989	2	0	1	0	1	91.844	2	NC	1
557		13	max	.019	3	.613	3	0	1	0	1	5133.881	15	NC	1
558			min	-.014	2	-.828	2	0	1	0	1	105.829	2	NC	1
559		14	max	.019	3	.473	3	0	1	0	1	6210.484	15	NC	1
560			min	-.013	2	-.628	2	0	1	0	1	130.336	2	NC	1
561		15	max	.018	3	.317	3	0	1	0	1	8077.62	15	NC	1
562			min	-.013	2	-.412	2	0	1	0	1	173.904	2	NC	1
563		16	max	.018	3	.159	3	0	1	0	1	NC	15	NC	1
564			min	-.013	2	-.201	2	0	1	0	1	258.042	2	NC	1
565		17	max	.017	3	.012	3	0	1	0	1	NC	5	NC	1
566			min	-.013	2	-.017	2	0	1	0	1	446.307	2	NC	1
567		18	max	.017	3	.121	2	0	1	0	1	NC	5	NC	1
568			min	-.013	2	-.112	3	0	1	0	1	991.783	2	NC	1
569		19	max	.017	3	.233	2	0	1	0	1	NC	1	NC	1
570			min	-.013	2	-.224	3	0	1	0	1	NC	1	NC	1
571	M9	1	max	.008	3	.132	2	0	15	2.249e-2	3	NC	1	NC	1
572			min	-.004	2	-.03	3	0	1	-1.095e-2	1	NC	1	NC	1
573		2	max	.008	3	.064	2	.004	1	1.112e-2	3	NC	4	NC	1
574			min	-.004	2	-.014	3	0	15	-5.357e-3	2	1696.353	2	NC	1
575		3	max	.008	3	.011	3	.005	1	1.191e-4	3	NC	5	NC	1
576			min	-.004	2	-.009	2	0	15	-3.496e-5	10	818.966	2	NC	1
577		4	max	.008	3	.053	3	.005	1	4.536e-3	3	NC	5	NC	1
578			min	-.004	2	-.091	2	0	15	-4.167e-3	2	518.254	2	NC	1
579		5	max	.007	3	.105	3	.003	1	8.953e-3	3	NC	5	NC	1
580			min	-.004	2	-.176	2	0	15	-8.306e-3	2	374.814	2	NC	1
581		6	max	.007	3	.161	3	.001	1	1.337e-2	3	NC	15	NC	1
582			min	-.004	2	-.258	2	0	15	-1.244e-2	2	295.678	2	NC	1
583		7	max	.007	3	.215	3	0	3	1.779e-2	3	NC	15	NC	1
584			min	-.004	2	-.331	2	0	1	-1.658e-2	2	248.907	2	NC	1
585		8	max	.007	3	.26	3	0	15	2.22e-2	3	9703.718	15	NC	1
586			min	-.004	2	-.39	2	0	1	-2.072e-2	2	221.216	2	NC	1
587		9	max	.007	3	.289	3	0	1	2.249e-2	3	9077.337	15	NC	1
588			min	-.004	2	-.426	2	0	15	-2.345e-2	2	206.791	2	NC	1
589		10	max	.007	3	.299	3	0	15	2.004e-2	3	8886.238	15	NC	1
590			min	-.004	2	-.439	2	0	1	-2.523e-2	2	202.56	2	NC	1
591		11	max	.007	3	.292	3	0	15	1.759e-2	3	9077.01	15	NC	1
592			min	-.004	2	-.426	2	0	1	-2.701e-2	2	207.489	2	NC	1
593		12	max	.006	3	.268	3	0	1	1.492e-2	3	9703.016	15	NC	1
594			min	-.004	2	-.388	2	0	15	-2.603e-2	2	223.323	2	NC	1
595		13	max	.006	3	.228	3	0	1	1.195e-2	3	NC	15	NC	1
596			min	-.004	2	-.328	2	0	15	-2.087e-2	2	253.999	2	NC	1
597		14	max	.006	3	.177	3	0	15	8.971e-3	3	NC	15	NC	1
598			min	-.003	2	-.252	2	-.001	1	-1.571e-2	2	306.495	2	NC	1
599		15	max	.006	3	.12	3	0	15	5.996e-3	3	NC	5	NC	1
600			min	-.003	2	-.168	2	-.003	1	-1.055e-2	2	396.946	2	NC	1
601		16	max	.006	3	.061	3	0	15	3.02e-3	3	NC	5	NC	1
602			min	-.003	2	-.083	2	-.005	1	-5.387e-3	2	564.618	2	NC	1
603		17	max	.006	3	.004	3	0	15	4.359e-5	3	NC	5	NC	1
604			min	-.003	2	-.006	2	-.005	1	-3.902e-4	1	923.363	2	NC	1
605		18	max	.006	3	.059	2	0	15	3.541e-3	3	NC	4	NC	1
606			min	-.003	2	-.047	3	-.004	1	-8.504e-3	2	1960.948	2	NC	1
607		19	max	.006	3	.116	2	0	1	7.189e-3	3	NC	1	NC	1
608			min	-.003	2	-.094	3	0	15	-1.709e-2	2	NC	1	NC	1



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Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

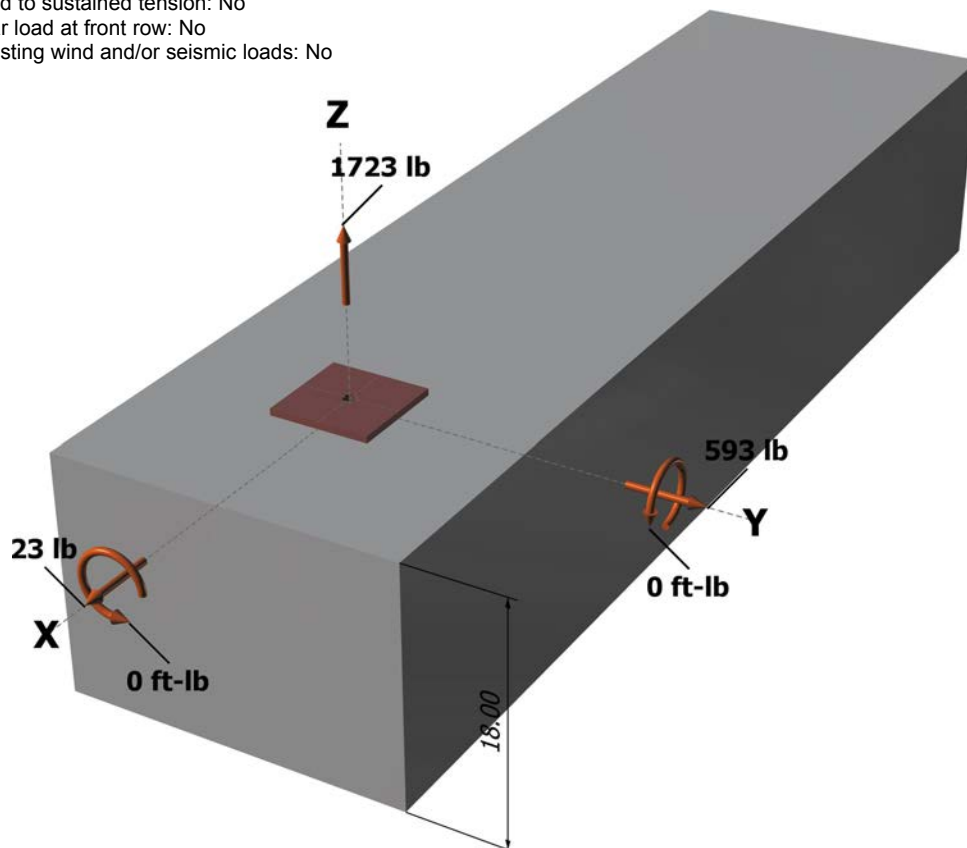
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263





# Anchor Designer™ Software Version 2.4.5673.0

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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	1723.0	23.0	593.0	593.4
Sum	1723.0	23.0	593.0	593.4

Maximum concrete compression strain (%): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 1723  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	5.247	10215

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cb}$ (lb)
220.36	247.75	0.967	1.00	1.000	10215	0.65	5710

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_a$ (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

**Shear perpendicular to edge in y-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	0.925	1.000	1.000	6947	0.70	3934

**Shear perpendicular to edge in x-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	0.878	1.000	1.000	8282	0.70	3018

**Shear parallel to edge in x-direction:**

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	7.00	6947

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
192.89	220.50	1.000	1.000	1.000	6947	0.70	8508

**Shear parallel to edge in y-direction:**

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
165.27	278.72	1.000	1.000	1.000	8282	0.70	6875

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$	$\phi V_{cp}$ (lb)
220.36	247.75	0.967	1.000	1.000	10215	8785	0.70	12298

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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Address:			
Phone:			
E-mail:			

## 11. Results

### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	1723	6071	0.28	Pass	
Concrete breakout	1723	5710	0.30	Pass	
<b>Adhesive</b>	<b>1723</b>	<b>5365</b>	<b>0.32</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>593</b>	<b>3156</b>	<b>0.19</b>	<b>Pass (Governs)</b>	
T Concrete breakout y+	593	3934	0.15	Pass	
T Concrete breakout x+	23	3018	0.01	Pass	
Concrete breakout y+	23	8508	0.00	Pass	
Concrete breakout x+	593	6875	0.09	Pass	
Concrete breakout, combined	-	-	0.15	Pass	
Pryout	593	12298	0.05	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.32	0.00	32.1 %	1.0	Pass

**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

## 12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Engineer:	HCV	Page:	1/5
Project:	Standard PVMax - Worst Case, 37-42 Inch Width		
Address:			
Phone:			
E-mail:			

### 1. Project information

Customer company:  
Customer contact name:  
Customer e-mail:  
Comment:

Project description:  
Location:  
Fastening description:

### 2. Input Data & Anchor Parameters

#### General

Design method: ACI 318-05  
Units: Imperial units

#### Anchor Information:

Anchor type: Bonded anchor  
Material: A193 Grade B8/B8M (304/316SS)  
Diameter (inch): 0.500  
Effective Embedment depth,  $h_{ef}$  (inch): 6.000  
Code report: IAPMO UES ER-263  
Anchor category: -  
Anchor ductility: Yes  
 $h_{min}$  (inch): 8.50  
 $C_{ac}$  (inch): 9.67  
 $C_{min}$  (inch): 1.75  
 $S_{min}$  (inch): 3.00

#### Load and Geometry

Load factor source: ACI 318 Section 9.2  
Load combination: not set  
Seismic design: No  
Anchors subjected to sustained tension: No  
Apply entire shear load at front row: No  
Anchors only resisting wind and/or seismic loads: No

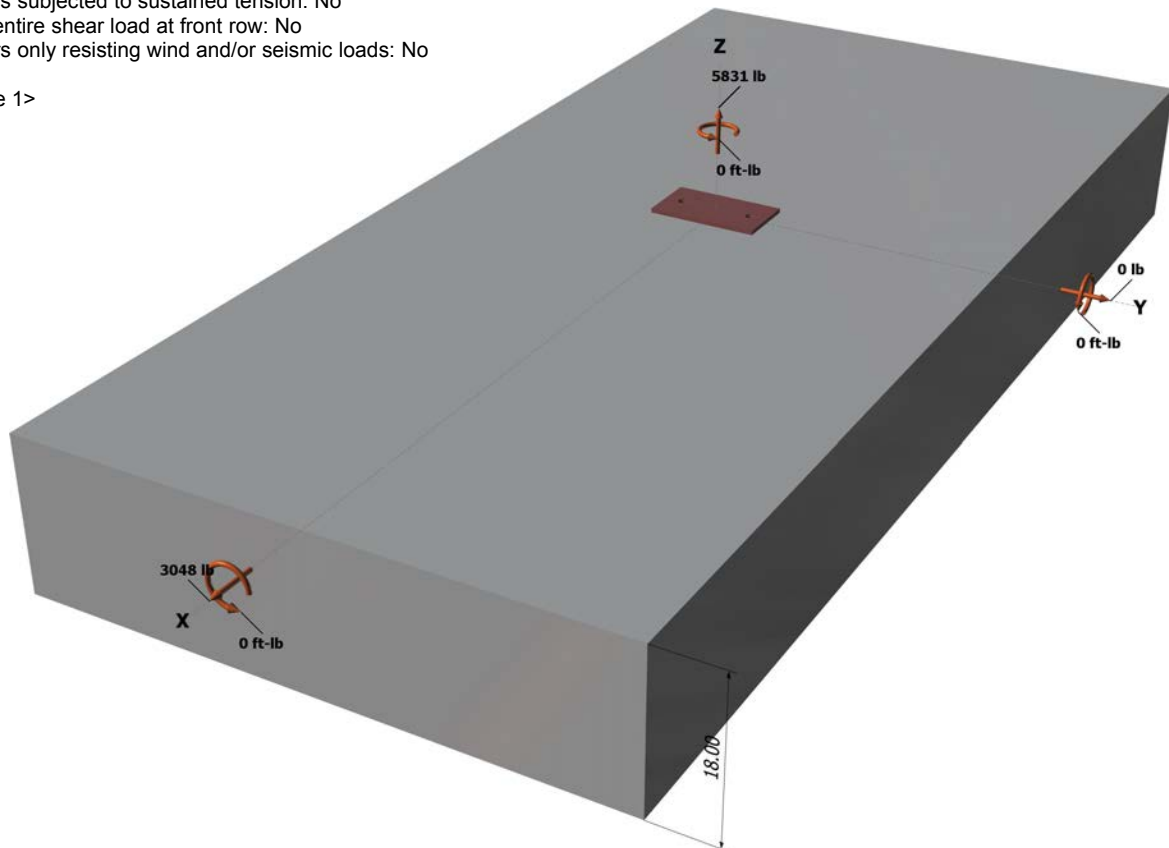
#### Base Material

Concrete: Normal-weight  
Concrete thickness,  $h$  (inch): 18.00  
State: Cracked  
Compressive strength,  $f'_c$  (psi): 2500  
 $\Psi_{c,v}$ : 1.0  
Reinforcement condition: B tension, B shear  
Supplemental reinforcement: Not applicable  
Reinforcement provided at corners: No  
Do not evaluate concrete breakout in tension: No  
Do not evaluate concrete breakout in shear: No  
Hole condition: Dry concrete  
Inspection: Periodic  
Temperature range, Short/Long: 110/75°F  
Ignore 6do requirement: Not applicable  
Build-up grout pad: No

#### Base Plate

Length x Width x Thickness (inch): 4.00 x 7.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

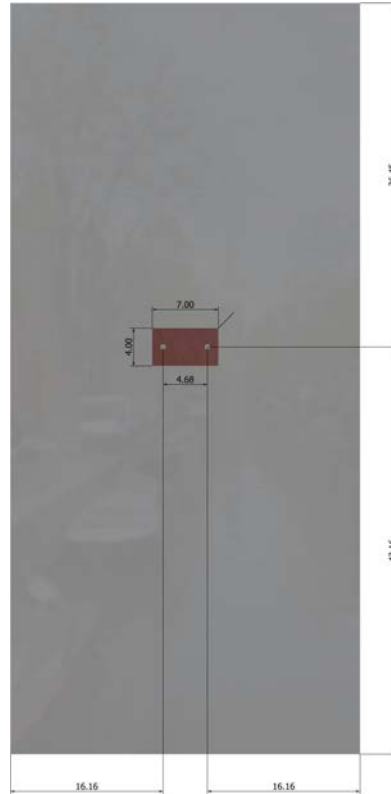
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<Figure 2>



**Recommended Anchor**

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)  
Code Report: IAPMO UES ER-263







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## 3. Resulting Anchor Forces

Anchor	Tension load, $N_{ua}$ (lb)	Shear load x, $V_{uax}$ (lb)	Shear load y, $V_{uay}$ (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	2915.5	1524.0	0.0	1524.0
2	2915.5	1524.0	0.0	1524.0
Sum	5831.0	3048.0	0.0	3048.0

Maximum concrete compression strain (‰): 0.00  
Maximum concrete compression stress (psi): 0  
Resultant tension force (lb): 5831  
Resultant compression force (lb): 0  
Eccentricity of resultant tension forces in x-axis,  $e'_{Nx}$  (inch): 0.00  
Eccentricity of resultant tension forces in y-axis,  $e'_{Ny}$  (inch): 0.00  
Eccentricity of resultant shear forces in x-axis,  $e'_{Vx}$  (inch): 0.00  
Eccentricity of resultant shear forces in y-axis,  $e'_{Vy}$  (inch): 0.00

<Figure 3>



## 4. Steel Strength of Anchor in Tension (Sec. D.5.1)

$N_{sa}$ (lb)	$\phi$	$\phi N_{sa}$ (lb)
8095	0.75	6071

## 5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

$k_c$	$\lambda$	$f'_c$ (psi)	$h_{ef}$ (in)	$N_b$ (lb)
17.0	1.00	2500	6.000	12492

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$\phi$	$\phi N_{cbg}$ (lb)
408.24	324.00	1.000	1.000	1.00	1.000	12492	0.65	10231

## 6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	$K_{sat}$	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	$d_a$ (in)	$h_{ef}$ (in)	$N_{a0}$ (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$\phi$	$\phi N_{ag}$ (lb)
158.66	109.66	1.000	1.043	1.000	1.000	9755	0.55	8093

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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### 8. Steel Strength of Anchor in Shear (Sec. D.6.1)

$V_{sa}$ (lb)	$\phi_{grout}$	$\phi$	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

### 9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{bx}$ (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbgx} = \phi (A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 \& Eq. D-22)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{bx}$ (lb)	$\phi$	$\phi V_{cbgx}$ (lb)
666.00	648.00	1.000	0.969	1.000	1.000	15593	0.70	10875

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

$l_e$ (in)	$d_a$ (in)	$\lambda$	$f'_c$ (psi)	$c_{a1}$ (in)	$V_{by}$ (lb)
4.00	0.50	1.00	2500	16.16	24369

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) \& Eq. D-21)}$$

$A_{Vc}$ (in <sup>2</sup> )	$A_{Vco}$ (in <sup>2</sup> )	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	$V_{by}$ (lb)	$\phi$	$\phi V_{cbx}$ (lb)
872.64	1175.16	1.000	1.000	1.000	24369	0.70	25334

### 10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

$k_{cp}$	$A_{Na}$ (in <sup>2</sup> )	$A_{Na0}$ (in <sup>2</sup> )	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	$N_{a0}$ (lb)	$N_a$ (lb)
2.0	158.66	109.66	1.000	1.043	1.000	1.000	9755	14715

$A_{Nc}$ (in <sup>2</sup> )	$A_{Nco}$ (in <sup>2</sup> )	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	$N_b$ (lb)	$N_{cb}$ (lb)	$\phi$
408.24	324.00	1.000	1.000	1.000	1.000	12492	15740	0.70

$$\phi V_{cpq} \text{ (lb)}$$

20601

### 11. Results

#### Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, $N_{ua}$ (lb)	Design Strength, $\phi N_n$ (lb)	Ratio	Status	
Steel	2916	6071	0.48	Pass	
Concrete breakout	5831	10231	0.57	Pass	
<b>Adhesive</b>	<b>5831</b>	<b>8093</b>	<b>0.72</b>	<b>Pass (Governs)</b>	
Shear	Factored Load, $V_{ua}$ (lb)	Design Strength, $\phi V_n$ (lb)	Ratio	Status	
<b>Steel</b>	<b>1524</b>	<b>3156</b>	<b>0.48</b>	<b>Pass (Governs)</b>	
T Concrete breakout x+	3048	10875	0.28	Pass	
Concrete breakout y-	1524	25334	0.06	Pass	
Pryout	3048	20601	0.15	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.72	0.48	120.3 %	1.2	Pass
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**AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.**

#### **12. Warnings**

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.