

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	30° Tilt w/ Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum		Minimum
Height =	1700 mm	Height =	1550 mm
Width =	1050 mm	Width =	970 mm
Dead Load =	3.00 psf	Dead Load =	1.75 psf

Modules Per Row = 1
Module Tilt = 30°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	16.49 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.73	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	85 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 11.34 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.15	(Pressure)
$C_{f+ BOTTOM}$ =	1.85	
$C_{f- TOP}$ =	-2.3	(Suction)
$C_{f- BOTTOM}$ =	-1.1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads

S_S =	2.50	R = 1.25
S_{DS} =	1.67	C_s = 0.8
S_1 =	1.00	ρ = 1.3
S_{D1} =	1.00	Ω = 1.25
T_a =	0.04	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

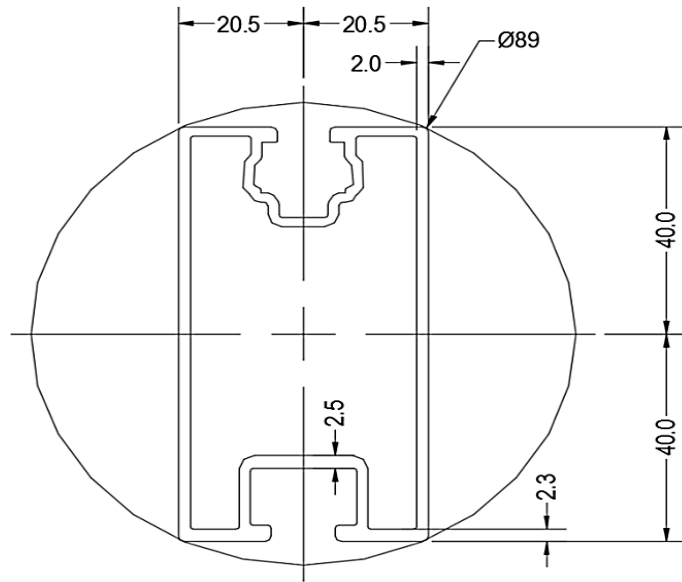
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

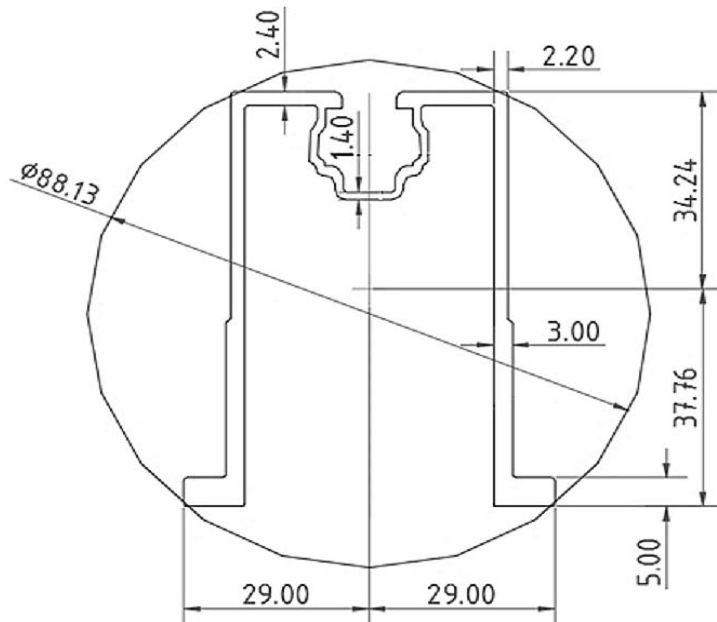
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	96 in
ΦF_{ty} STRONG-AXIS =	28.76 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	0.942 k-ft
M_z =	0.251 k-ft
$M_{y \text{ allowable}}$ =	1.787 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	83%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.84 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.596 k-ft
M_z =	0.000 k-ft
P_n =	0.316 k
$M_{y \text{ allowable}}$ =	1.464 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	43%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	-0.055 k-ft
P_n =	0.276 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	15%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.624 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	16%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	39.29 in
$\Phi F_{ty \text{ AXIAL}}$ =	10.06 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.09 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.031 k
$M_{y \text{ allowable}}$ =	0.408 k-ft
$M_{z \text{ allowable}}$ =	0.408 k-ft
$P_{n \text{ allowable}}$ =	5.050 k
Utilization =	<u>20%</u>



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.008 k-ft
P_n =	0.267 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	<u>20%</u>



A cross brace kit is required every 10 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

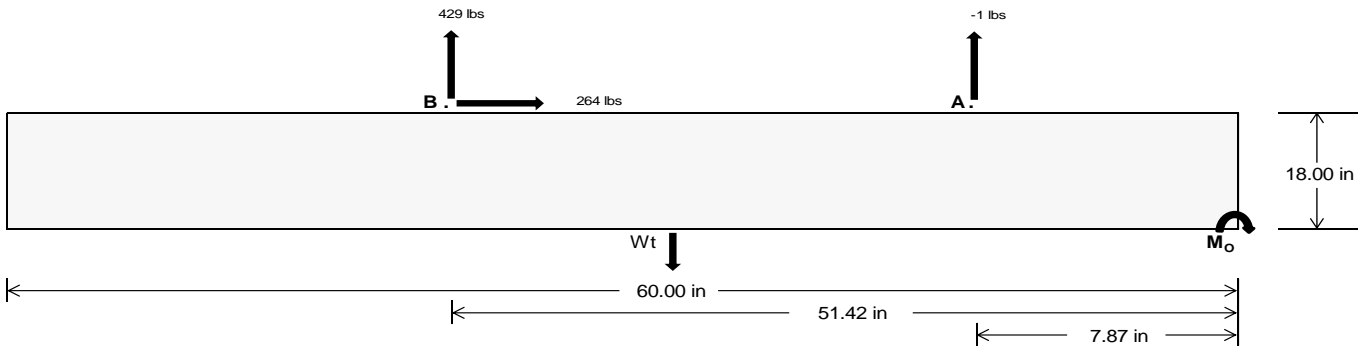
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	<u>2.11</u>	<u>1787.02</u>	k
Compressive Load =	<u>1597.09</u>	<u>1387.51</u>	k
Lateral Load =	<u>44.57</u>	<u>1100.66</u>	k
Moment (Weak Axis) =	<u>0.07</u>	<u>0.00</u>	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 26796.0$ in-lbs
Resisting Force Required = 893.20 lbs
S.F. = 1.67
Weight Required = 1488.67 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 264.44 lbs
Friction = 0.4
Weight Required = 661.10 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 264.44 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	631 lbs	631 lbs	631 lbs	631 lbs	444 lbs	444 lbs	444 lbs	444 lbs	750 lbs	750 lbs	750 lbs	750 lbs	2 lbs	2 lbs	2 lbs	2 lbs
F_B	443 lbs	443 lbs	443 lbs	443 lbs	570 lbs	570 lbs	570 lbs	570 lbs	719 lbs	719 lbs	719 lbs	719 lbs	-857 lbs	-857 lbs	-857 lbs	-857 lbs
F_V	76 lbs	76 lbs	76 lbs	76 lbs	484 lbs	484 lbs	484 lbs	484 lbs	414 lbs	414 lbs	414 lbs	414 lbs	-529 lbs	-529 lbs	-529 lbs	-529 lbs
P_{total}	3068 lbs	3159 lbs	3249 lbs	3340 lbs	3008 lbs	3098 lbs	3189 lbs	3280 lbs	3462 lbs	3553 lbs	3644 lbs	3734 lbs	341 lbs	395 lbs	449 lbs	504 lbs
M	488 lbs-ft	488 lbs-ft	488 lbs-ft	488 lbs-ft	527 lbs-ft	527 lbs-ft	527 lbs-ft	527 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft	720 lbs-ft	740 lbs-ft	740 lbs-ft	740 lbs-ft	740 lbs-ft
e	0.16 ft	0.15 ft	0.15 ft	0.15 ft	0.18 ft	0.17 ft	0.17 ft	0.16 ft	0.21 ft	0.20 ft	0.20 ft	0.19 ft	2.17 ft	1.87 ft	1.65 ft	1.47 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	270.9 psf	268.5 psf	266.4 psf	264.5 psf	259.2 psf	257.4 psf	255.7 psf	254.2 psf	283.5 psf	280.6 psf	278.0 psf	275.6 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	398.5 psf	390.7 psf	383.4 psf	376.8 psf	397.1 psf	389.3 psf	382.1 psf	375.5 psf	472.0 psf	460.9 psf	450.8 psf	441.4 psf	379.1 psf	219.5 psf	175.7 psf	156.4 psf

Maximum Bearing Pressure = 472 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Seismic Design

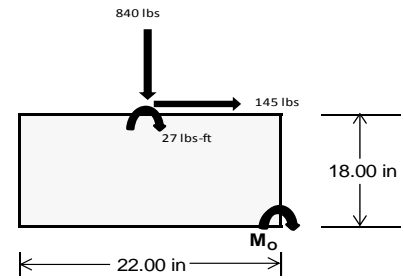
Overturning Check

$M_o = 526.2 \text{ ft-lbs}$
 Resisting Force Required = 573.99 lbs
 S.F. = 1.67
 Weight Required = 956.64 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	153 lbs	177 lbs	93 lbs	356 lbs	840 lbs	309 lbs	87 lbs	6 lbs	31 lbs
F_v	23 lbs	191 lbs	24 lbs	15 lbs	145 lbs	19 lbs	24 lbs	190 lbs	24 lbs
P_{total}	2621 lbs	2645 lbs	2561 lbs	2706 lbs	3190 lbs	2659 lbs	809 lbs	727 lbs	752 lbs
M	67 lbs-ft	323 lbs-ft	74 lbs-ft	43 lbs-ft	244 lbs-ft	58 lbs-ft	69 lbs-ft	323 lbs-ft	72 lbs-ft
e	0.03 ft	0.12 ft	0.03 ft	0.02 ft	0.08 ft	0.02 ft	0.09 ft	0.44 ft	0.10 ft
$L/6$	0.31 ft	1.59 ft	1.78 ft	1.80 ft	1.68 ft	1.79 ft	1.66 ft	0.95 ft	1.64 ft
f_{min}	262.0 sqft	173.1 sqft	253.1 sqft	279.9 sqft	260.9 sqft	269.5 sqft	63.7 sqft	-35.9 sqft	56.3 sqft
f_{max}	309.9 psf	403.9 psf	305.6 psf	310.5 psf	435.1 psf	310.6 psf	112.8 psf	194.6 psf	107.9 psf



Maximum Bearing Pressure = 435 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.386 k
Allowable Uplift =	1.214 k
Utilization =	<u>32%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.088 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.229 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>22%</u>

Diagonal Strut

Maximum Axial Load =	0.624 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>11%</u>



Rear Strut

Maximum Axial Load =	1.173 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.267 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>3%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

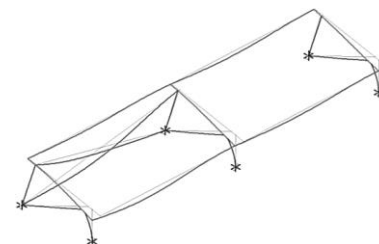
7. SEISMIC DESIGN

7.1 Seismic Drift

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	32.32 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.646 in
Max Drift, Δ_{MAX} =	0.116 in
	<u>0.116 ≤ 0.646. OK.</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 96.00 \text{ in}$$

$$J = 0.427$$

$$200.222$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 96.00 \text{ in}$$

$$J = 0.427$$

$$217.57$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.787 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi k_2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.38 \\
 &20.9468 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.38 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.8 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.464 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k_2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p * b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 39.29 \text{ in}$$

$$J = 103.073$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.1$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.1 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.408 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.68476 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.81587 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 10.0603 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

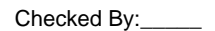
3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 10.06 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 5.05 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.

RISA-3D Version 13.0.0 [\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\...\\.\\PVMMini 60 Cell 1V 30° 85mph 30psf 8ft 7-05.r3d]Page 20



Company : Schletter, Inc.
Designer : HCV
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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
29		15	max	316.631	1	-.016	15	.649	1	0	12	.001	4	0	15
30			min	-359.461	3	-.073	3	-.367	5	-.001	1	0	3	0	6
31		16	max	316.757	1	-.028	15	.649	1	0	12	.001	1	0	15
32			min	-359.366	3	-.116	4	-.481	5	-.001	1	0	3	0	6
33		17	max	316.883	1	-.04	15	.649	1	0	12	.002	1	0	15
34			min	-359.272	3	-.167	4	-.596	5	-.001	1	0	3	0	6
35		18	max	317.008	1	-.052	15	.649	1	0	12	.002	1	0	15
36			min	-359.178	3	-.218	4	-.71	5	-.001	1	0	3	0	6
37		19	max	317.134	1	-.064	15	.649	1	0	12	.002	1	0	15
38			min	-359.083	3	-.269	4	-.825	5	-.001	1	0	3	0	6
39	M3	1	max	148.345	2	1.755	6	-.04	12	0	5	.002	1	0	6
40			min	-174.51	3	.412	15	-1.48	4	0	1	0	12	0	15
41		2	max	148.276	2	1.578	6	-.04	12	0	5	.002	1	0	2
42			min	-174.562	3	.371	15	-1.346	4	0	1	0	12	0	12
43		3	max	148.206	2	1.401	6	-.04	12	0	5	.002	1	0	2
44			min	-174.614	3	.329	15	-1.212	4	0	1	0	15	0	3
45		4	max	148.137	2	1.225	6	-.04	12	0	5	.002	1	0	15
46			min	-174.666	3	.287	15	-1.079	4	0	1	0	5	0	4
47		5	max	148.068	2	1.048	6	-.04	12	0	5	.002	1	0	15
48			min	-174.718	3	.246	15	-.945	4	0	1	0	5	0	4
49		6	max	147.998	2	.871	6	-.04	12	0	5	.002	1	0	15
50			min	-174.77	3	.204	15	-.811	4	0	1	0	5	0	4
51		7	max	147.929	2	.694	6	-.04	12	0	5	.001	1	0	15
52			min	-174.822	3	.163	15	-.726	1	0	1	0	5	0	4
53		8	max	147.86	2	.517	6	-.04	12	0	5	.001	1	0	15
54			min	-174.874	3	.121	15	-.726	1	0	1	0	5	-.001	4
55		9	max	147.79	2	.34	6	-.04	12	0	5	.001	1	0	15
56			min	-174.926	3	.08	15	-.726	1	0	1	0	5	-.001	4
57		10	max	147.721	2	.164	6	-.04	12	0	5	0	1	0	15
58			min	-174.978	3	.038	15	-.726	1	0	1	0	5	-.001	4
59		11	max	147.652	2	.015	2	.014	5	0	5	0	1	0	15
60			min	-175.03	3	-.038	3	-.726	1	0	1	0	5	-.001	4
61		12	max	147.582	2	-.045	15	.148	5	0	5	0	1	0	15
62			min	-175.082	3	-.19	4	-.726	1	0	1	0	5	-.001	4
63		13	max	147.513	2	-.087	15	.281	5	0	5	0	1	0	15
64			min	-175.134	3	-.367	4	-.726	1	0	1	0	5	-.001	4
65		14	max	147.444	2	-.128	15	.415	5	0	5	0	1	0	15
66			min	-175.186	3	-.544	4	-.726	1	0	1	0	5	-.001	4
67		15	max	147.374	2	-.17	15	.549	5	0	5	0	1	0	15
68			min	-175.238	3	-.721	4	-.726	1	0	1	0	5	0	4
69		16	max	147.305	2	-.211	15	.682	5	0	5	0	1	0	15
70			min	-175.29	3	-.898	4	-.726	1	0	1	0	5	0	4
71		17	max	147.236	2	-.253	15	.816	5	0	5	0	12	0	15
72			min	-175.342	3	-1.074	4	-.726	1	0	1	0	4	0	4
73		18	max	147.167	2	-.295	15	.95	5	0	5	0	12	0	15
74			min	-175.394	3	-1.251	4	-.726	1	0	1	0	1	0	4
75		19	max	147.097	2	-.336	15	1.083	5	0	5	0	5	0	1
76			min	-175.446	3	-1.428	4	-.726	1	0	1	0	1	0	1
77	M4	1	max	471.621	1	0	1	-.202	12	0	1	0	5	0	1
78			min	7.366	12	0	1	-33.748	4	0	1	0	1	0	1
79		2	max	471.686	1	0	1	-.202	12	0	1	0	12	0	1
80			min	7.399	12	0	1	-33.805	4	0	1	-.003	4	0	1
81		3	max	471.75	1	0	1	-.202	12	0	1	0	12	0	1
82			min	7.431	12	0	1	-33.861	4	0	1	-.006	4	0	1
83		4	max	471.815	1	0	1	-.202	12	0	1	0	12	0	1
84			min	7.463	12	0	1	-33.917	4	0	1	-.009	4	0	1
85		5	max	471.88	1	0	1	-.202	12	0	1	0	12	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
143	15	max	1030.073	1	.051	2	.173	1	0	1	.001	4	0	12
144		min	-1171.406	3	-.151	3	-.41	5	0	5	0	3	0	2
145	16	max	1030.199	1	.011	2	.173	1	0	1	.001	4	0	12
146		min	-1171.312	3	-.181	3	-.524	5	0	5	0	3	0	2
147	17	max	1030.325	1	-.029	2	.173	1	0	1	.001	4	0	12
148		min	-1171.217	3	-.211	3	-.638	5	0	5	0	3	0	2
149	18	max	1030.45	1	-.061	15	.173	1	0	1	0	4	0	3
150		min	-1171.123	3	-.241	3	-.753	5	0	5	0	3	0	2
151	19	max	1030.576	1	-.073	15	.173	1	0	1	0	4	0	3
152		min	-1171.029	3	-.28	4	-.867	5	0	5	0	3	0	2
153	M7	1	max	623.952	2	1.774	.016	1	0	2	0	4	0	2
154		min	-544.359	3	.422	15	-1.348	5	0	3	0	3	0	3
155	2	max	623.883	2	1.597	4	.016	1	0	2	0	4	0	2
156		min	-544.411	3	.381	15	-1.214	5	0	3	0	3	0	3
157	3	max	623.814	2	1.42	4	.016	1	0	2	0	2	0	2
158		min	-544.463	3	.339	15	-1.08	5	0	3	0	3	0	3
159	4	max	623.744	2	1.243	4	.016	1	0	2	0	2	0	2
160		min	-544.515	3	.298	15	-.947	5	0	3	0	3	0	3
161	5	max	623.675	2	1.066	4	.016	1	0	2	0	2	0	15
162		min	-544.567	3	.256	15	-.813	5	0	3	0	5	0	3
163	6	max	623.606	2	.89	4	.016	1	0	2	0	2	0	15
164		min	-544.619	3	.215	15	-.679	5	0	3	0	5	0	3
165	7	max	623.536	2	.713	4	.016	1	0	2	0	2	0	15
166		min	-544.671	3	.173	15	-.546	5	0	3	0	5	0	6
167	8	max	623.467	2	.536	4	.016	1	0	2	0	2	0	15
168		min	-544.723	3	.131	15	-.412	5	0	3	0	5	-.001	6
169	9	max	623.398	2	.36	2	.016	1	0	2	0	2	0	15
170		min	-544.775	3	.066	12	-.278	5	0	3	0	5	-.001	6
171	10	max	623.328	2	.223	2	.016	1	0	2	0	2	0	15
172		min	-544.827	3	-.01	3	-.145	5	0	3	0	5	-.001	6
173	11	max	623.259	2	.085	2	.016	1	0	2	0	2	0	15
174		min	-544.879	3	-.114	3	-.011	5	0	3	0	5	-.001	6
175	12	max	623.19	2	-.035	15	.126	4	0	2	0	2	0	15
176		min	-544.931	3	-.217	3	-.004	10	0	3	0	5	-.001	6
177	13	max	623.121	2	-.076	15	.26	4	0	2	0	2	0	15
178		min	-544.983	3	-.349	6	-.004	10	0	3	0	5	-.001	6
179	14	max	623.051	2	-.118	15	.393	4	0	2	0	2	0	15
180		min	-545.035	3	-.525	6	-.004	10	0	3	0	5	-.001	6
181	15	max	622.982	2	-.16	15	.527	4	0	2	0	2	0	15
182		min	-545.087	3	-.702	6	-.004	10	0	3	0	5	0	6
183	16	max	622.913	2	-.201	15	.661	4	0	2	0	2	0	15
184		min	-545.139	3	-.879	6	-.004	10	0	3	0	5	0	6
185	17	max	622.843	2	-.243	15	.794	4	0	2	0	2	0	15
186		min	-545.191	3	-1.056	6	-.004	10	0	3	0	5	0	6
187	18	max	622.774	2	-.284	15	.928	4	0	2	0	2	0	15
188		min	-545.243	3	-1.233	6	-.004	10	0	3	0	5	0	6
189	19	max	622.705	2	-.326	15	1.062	4	0	2	0	14	0	1
190		min	-545.295	3	-1.41	6	-.004	10	0	3	0	3	0	1
191	M8	1	max	1227.368	1	0	.917	1	0	1	0	4	0	1
192		min	-2.494	3	0	1	-33.716	4	0	1	0	1	0	1
193	2	max	1227.432	1	0	1	.917	1	0	1	0	1	0	1
194		min	-2.445	3	0	1	-33.772	4	0	1	-.003	4	0	1
195	3	max	1227.497	1	0	1	.917	1	0	1	0	1	0	1
196		min	-2.397	3	0	1	-33.828	4	0	1	-.006	4	0	1
197	4	max	1227.562	1	0	1	.917	1	0	1	0	1	0	1
198		min	-2.348	3	0	1	-33.884	4	0	1	-.009	4	0	1
199	5	max	1227.626	1	0	1	.917	1	0	1	0	1	0	1



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
200			min	-2.3	3	0	1	-33.94	4	0	1	-.012	4	0	1
201		6	max	1227.691	1	0	1	.917	1	0	1	0	1	0	1
202			min	-2.251	3	0	1	-33.997	4	0	1	-.015	4	0	1
203		7	max	1227.756	1	0	1	.917	1	0	1	0	1	0	1
204			min	-2.203	3	0	1	-34.053	4	0	1	-.018	4	0	1
205		8	max	1227.821	1	0	1	.917	1	0	1	0	1	0	1
206			min	-2.154	3	0	1	-34.109	4	0	1	-.021	4	0	1
207		9	max	1227.885	1	0	1	.917	1	0	1	0	1	0	1
208			min	-2.106	3	0	1	-34.165	4	0	1	-.024	4	0	1
209		10	max	1227.95	1	0	1	.917	1	0	1	0	1	0	1
210			min	-2.057	3	0	1	-34.221	4	0	1	-.027	4	0	1
211		11	max	1228.015	1	0	1	.917	1	0	1	0	1	0	1
212			min	-2.009	3	0	1	-34.277	4	0	1	-.03	4	0	1
213		12	max	1228.079	1	0	1	.917	1	0	1	0	1	0	1
214			min	-1.96	3	0	1	-34.333	4	0	1	-.033	4	0	1
215		13	max	1228.144	1	0	1	.917	1	0	1	0	1	0	1
216			min	-1.911	3	0	1	-34.389	4	0	1	-.037	4	0	1
217		14	max	1228.209	1	0	1	.917	1	0	1	.001	1	0	1
218			min	-1.863	3	0	1	-34.445	4	0	1	-.04	4	0	1
219		15	max	1228.273	1	0	1	.917	1	0	1	.001	1	0	1
220			min	-1.814	3	0	1	-34.501	4	0	1	-.043	4	0	1
221		16	max	1228.338	1	0	1	.917	1	0	1	.001	1	0	1
222			min	-1.766	3	0	1	-34.557	4	0	1	-.046	4	0	1
223		17	max	1228.403	1	0	1	.917	1	0	1	.001	1	0	1
224			min	-1.717	3	0	1	-34.613	4	0	1	-.049	4	0	1
225		18	max	1228.468	1	0	1	.917	1	0	1	.001	1	0	1
226			min	-1.669	3	0	1	-34.669	4	0	1	-.052	4	0	1
227		19	max	1228.532	1	0	1	.917	1	0	1	.001	1	0	1
228			min	-1.62	3	0	1	-34.726	4	0	1	-.055	4	0	1
229	M10	1	max	331.785	1	.68	4	1.447	5	.001	1	0	1	0	1
230			min	-341.578	3	.172	15	-.235	1	-.002	5	0	5	0	1
231		2	max	331.911	1	.629	4	1.333	5	.001	1	0	1	0	15
232			min	-341.484	3	.16	15	-.235	1	-.002	5	0	3	0	4
233		3	max	332.037	1	.578	4	1.219	5	.001	1	0	4	0	15
234			min	-341.39	3	.148	15	-.235	1	-.002	5	0	3	0	4
235		4	max	332.163	1	.526	4	1.104	5	.001	1	0	4	0	15
236			min	-341.295	3	.136	15	-.235	1	-.002	5	0	3	0	4
237		5	max	332.289	1	.475	4	.99	5	.001	1	0	4	0	15
238			min	-341.201	3	.124	15	-.235	1	-.002	5	0	3	0	4
239		6	max	332.415	1	.424	4	.875	5	.001	1	0	4	0	15
240			min	-341.106	3	.112	15	-.235	1	-.002	5	0	3	0	4
241		7	max	332.54	1	.373	4	.761	5	.001	1	.001	4	0	15
242			min	-341.012	3	.1	15	-.235	1	-.002	5	0	3	0	4
243		8	max	332.666	1	.322	4	.646	5	.001	1	.001	4	0	15
244			min	-340.918	3	.088	15	-.235	1	-.002	5	0	3	0	4
245		9	max	332.792	1	.271	4	.532	5	.001	1	.001	4	0	15
246			min	-340.823	3	.076	15	-.235	1	-.002	5	0	3	0	4
247		10	max	332.918	1	.22	4	.418	5	.001	1	.001	4	0	15
248			min	-340.729	3	.064	15	-.235	1	-.002	5	0	3	0	4
249		11	max	333.044	1	.168	4	.303	5	.001	1	.001	4	0	15
250			min	-340.634	3	.048	12	-.235	1	-.002	5	0	3	0	4
251		12	max	333.17	1	.117	4	.189	5	.001	1	.001	4	0	15
252			min	-340.54	3	.028	12	-.235	1	-.002	5	0	3	0	4
253		13	max	333.296	1	.066	4	.074	5	.001	1	.002	4	0	15
254			min	-340.446	3	.007	1	-.235	1	-.002	5	0	1	0	4
255		14	max	333.421	1	.023	5	-.008	12	.001	1	.002	4	0	15
256			min	-340.351	3	-.033	1	-.235	1	-.002	5	0	1	0	4



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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
257		15	max	333.547	1	.004	5	-.008	12	.001	1	.001	4	0	15
258			min	-340.257	3	-.073	1	-.235	1	-.002	5	0	1	0	4
259		16	max	333.673	1	-.009	15	-.008	12	.001	1	.001	5	0	15
260			min	-340.162	3	-.113	1	-.303	4	-.002	5	0	1	0	4
261		17	max	333.799	1	-.021	15	-.008	12	.001	1	.001	5	0	15
262			min	-340.068	3	-.153	1	-.417	4	-.002	5	0	1	0	4
263		18	max	333.925	1	-.033	15	-.008	12	.001	1	.001	5	0	15
264			min	-339.974	3	-.193	1	-.532	4	-.002	5	0	1	0	4
265		19	max	334.051	1	-.045	15	-.008	12	.001	1	.001	5	0	15
266			min	-339.879	3	-.242	6	-.646	4	-.002	5	0	1	0	4
267	M11	1	max	148.115	2	1.746	6	.809	1	.002	4	.001	5	0	6
268			min	-175.154	3	.406	15	-1.199	5	0	10	-.002	1	0	15
269		2	max	148.046	2	1.569	6	.809	1	.002	4	0	5	0	1
270			min	-175.206	3	.364	15	-1.065	5	0	10	-.002	1	0	3
271		3	max	147.976	2	1.393	6	.809	1	.002	4	0	5	0	1
272			min	-175.258	3	.323	15	-.931	5	0	10	-.002	1	0	3
273		4	max	147.907	2	1.216	6	.809	1	.002	4	0	5	0	15
274			min	-175.31	3	.281	15	-.798	5	0	10	-.002	1	0	4
275		5	max	147.838	2	1.039	6	.809	1	.002	4	0	5	0	15
276			min	-175.362	3	.24	15	-.664	5	0	10	-.002	1	0	4
277		6	max	147.768	2	.862	6	.809	1	.002	4	0	5	0	15
278			min	-175.414	3	.198	15	-.53	5	0	10	-.001	1	0	4
279		7	max	147.699	2	.685	6	.809	1	.002	4	0	3	0	15
280			min	-175.466	3	.156	15	-.397	5	0	10	-.001	1	0	4
281		8	max	147.63	2	.508	6	.809	1	.002	4	0	3	0	15
282			min	-175.518	3	.115	15	-.263	5	0	10	-.001	1	-.001	4
283		9	max	147.561	2	.332	6	.809	1	.002	4	0	3	0	15
284			min	-175.57	3	.073	15	-.129	5	0	10	0	1	-.001	4
285		10	max	147.491	2	.155	6	.809	1	.002	4	0	3	0	15
286			min	-175.622	3	.032	15	.002	15	0	10	0	1	-.001	4
287		11	max	147.422	2	.016	1	.809	1	.002	4	0	3	0	15
288			min	-175.674	3	-.055	3	.011	12	0	10	0	1	-.001	4
289		12	max	147.353	2	-.051	15	.809	1	.002	4	0	3	0	15
290			min	-175.726	3	-.199	4	.011	12	0	10	0	1	-.001	4
291		13	max	147.283	2	-.093	15	.809	1	.002	4	0	3	0	15
292			min	-175.778	3	-.376	4	.011	12	0	10	0	1	-.001	4
293		14	max	147.214	2	-.135	15	.809	1	.002	4	0	5	0	15
294			min	-175.83	3	-.553	4	.011	12	0	10	0	1	-.001	4
295		15	max	147.145	2	-.176	15	.848	4	.002	4	0	4	0	15
296			min	-175.882	3	-.73	4	.011	12	0	10	0	10	0	4
297		16	max	147.075	2	-.218	15	.981	4	.002	4	0	4	0	15
298			min	-175.934	3	-.907	4	.011	12	0	10	0	10	0	4
299		17	max	147.006	2	-.259	15	1.115	4	.002	4	0	4	0	15
300			min	-175.986	3	-1.084	4	.011	12	0	10	0	10	0	4
301		18	max	146.937	2	-.301	15	1.249	4	.002	4	.001	4	0	15
302			min	-176.038	3	-1.26	4	.011	12	0	10	0	10	0	4
303		19	max	146.867	2	-.342	15	1.382	4	.002	4	.001	4	0	1
304			min	-176.09	3	-1.437	4	.011	12	0	10	0	10	0	1
305	M12	1	max	471.248	1	0	1	4.768	1	0	1	0	4	0	1
306			min	7.737	12	0	1	-30.797	5	0	1	0	3	0	1
307		2	max	471.313	1	0	1	4.768	1	0	1	0	1	0	1
308			min	7.769	12	0	1	-30.854	5	0	1	-.003	5	0	1
309		3	max	471.378	1	0	1	4.768	1	0	1	0	1	0	1
310			min	7.801	12	0	1	-30.91	5	0	1	-.005	5	0	1
311		4	max	471.442	1	0	1	4.768	1	0	1	.001	1	0	1
312			min	7.834	12	0	1	-30.966	5	0	1	-.008	5	0	1
313		5	max	471.507	1	0	1	4.768	1	0	1	.002	1	0	1



Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
371		15	max	96.853	1	5.157	9	-4.523	12	0	12	-.004	12	.191	2
372			min	-6.662	10	-26.414	2	-93.133	1	0	1	-.1	1	-.116	3
373		16	max	95.522	2	96.111	2	-4.564	12	0	1	-.005	12	.195	2
374			min	-5.286	3	-163.712	3	-93.729	1	0	5	-.121	1	-.111	3
375		17	max	95.662	2	95.869	2	-4.564	12	0	1	-.006	12	.174	2
376			min	-5.181	3	-163.893	3	-93.729	1	0	5	-.141	1	-.076	3
377		18	max	-6.693	12	373.14	2	-4.767	12	0	5	-.007	12	.095	2
378			min	-161.689	1	-155.413	3	-96.153	1	0	2	-.162	1	-.042	3
379		19	max	-6.623	12	372.898	2	-4.767	12	0	5	-.008	12	.014	2
380			min	-161.55	1	-155.594	3	-96.153	1	0	2	-.183	1	-.008	3
381	M5	1	max	356.5	1	1116.298	3	-.113	10	0	1	.048	4	.021	3
382			min	11.407	15	-1027.292	1	-38.658	3	0	5	0	10	-.023	1
383		2	max	356.64	1	1116.116	3	-.113	10	0	1	.041	4	.2	1
384			min	11.449	15	-1027.534	1	-38.658	3	0	5	-.004	3	-.221	3
385		3	max	275.741	3	7.087	9	4.395	3	0	3	.035	4	.418	1
386			min	-44.044	10	-85.264	2	-25.973	4	0	4	-.012	3	-.458	3
387		4	max	275.845	3	6.885	9	4.395	3	0	3	.029	4	.426	1
388			min	-43.928	10	-85.506	2	-25.731	4	0	4	-.011	3	-.447	3
389		5	max	275.95	3	6.684	9	4.395	3	0	3	.024	4	.433	1
390			min	-43.812	10	-85.748	2	-25.489	4	0	4	-.01	3	-.437	3
391		6	max	276.055	3	6.482	9	4.395	3	0	3	.018	4	.441	1
392			min	-43.695	10	-85.99	2	-25.247	4	0	4	-.009	3	-.426	3
393		7	max	276.159	3	6.281	9	4.395	3	0	3	.013	4	.449	1
394			min	-43.579	10	-86.232	2	-25.005	4	0	4	-.008	3	-.416	3
395		8	max	276.264	3	6.079	9	4.395	3	0	3	.007	4	.456	1
396			min	-43.463	10	-86.473	2	-24.763	4	0	4	-.007	3	-.405	3
397		9	max	276.369	3	5.878	9	4.395	3	0	3	.002	4	.468	2
398			min	-43.346	10	-86.715	2	-24.521	4	0	4	-.006	3	-.394	3
399		10	max	276.474	3	5.676	9	4.395	3	0	3	0	10	.487	2
400			min	-43.23	10	-86.957	2	-24.279	4	0	4	-.005	3	-.383	3
401		11	max	276.578	3	5.475	9	4.395	3	0	3	0	10	.505	2
402			min	-43.114	10	-87.199	2	-24.037	4	0	4	-.008	4	-.373	3
403		12	max	276.683	3	5.273	9	4.395	3	0	3	0	10	.524	2
404			min	-42.997	10	-87.441	2	-23.795	4	0	4	-.014	4	-.362	3
405		13	max	276.788	3	5.072	9	4.395	3	0	3	0	10	.543	2
406			min	-42.881	10	-87.683	2	-23.553	4	0	4	-.019	4	-.351	3
407		14	max	276.892	3	4.87	9	4.395	3	0	3	0	10	.562	2
408			min	-42.765	10	-87.924	2	-23.311	4	0	4	-.024	4	-.34	3
409		15	max	276.997	3	4.669	9	4.395	3	0	3	0	10	.581	2
410			min	-42.648	10	-88.166	2	-23.069	4	0	4	-.029	4	-.329	3
411		16	max	316.453	2	435.635	2	4.373	3	0	1	0	3	.596	2
412			min	-21.694	3	-506.825	3	-21.781	4	0	4	-.034	4	-.314	3
413		17	max	316.593	2	435.393	2	4.373	3	0	1	.001	3	.502	2
414			min	-21.589	3	-507.007	3	-21.539	4	0	4	-.039	4	-.204	3
415		18	max	-13.536	12	1226.902	2	4.018	3	0	4	.002	3	.239	2
416			min	-357.556	1	-509.343	3	-53.693	5	0	1	-.05	4	-.094	3
417		19	max	-13.466	12	1226.661	2	4.018	3	0	4	.003	3	.017	3
418			min	-357.416	1	-509.524	3	-53.451	5	0	1	-.062	4	-.027	2
419	M9	1	max	161.456	1	338.388	3	230.327	4	0	3	-.004	15	.012	1
420			min	4.657	15	-311.105	1	9.748	10	0	1	-.184	1	-.011	3
421		2	max	161.595	1	338.207	3	230.569	4	0	3	.039	5	.079	1
422			min	4.699	15	-311.347	1	9.748	10	0	1	-.157	1	-.084	3
423		3	max	95.096	1	7.547	9	87.862	1	0	1	.082	5	.145	1
424			min	-7.512	10	-23.526	2	-31.857	5	0	5	-.129	1	-.156	3
425		4	max	95.235	1	7.345	9	87.862	1	0	1	.075	5	.146	1
426			min	-7.396	10	-23.767	2	-31.615	5	0	5	-.11	1	-.153	3
427		5	max	95.375	1	7.144	9	87.862	1	0	1	.068	5	.147	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
428			min	-7.279	10	-24.009	2	-31.373	5	0	5	-.09	1	-.15	3
429		6	max	95.515	1	6.942	9	87.862	1	0	1	.062	5	.148	1
430			min	-7.163	10	-24.251	2	-31.131	5	0	5	-.071	1	-.147	3
431		7	max	95.654	1	6.741	9	87.862	1	0	1	.055	5	.149	1
432			min	-7.047	10	-24.493	2	-30.889	5	0	5	-.052	1	-.143	3
433		8	max	95.794	1	6.539	9	87.862	1	0	1	.048	5	.152	2
434			min	-6.93	10	-24.735	2	-30.647	5	0	5	-.033	1	-.14	3
435		9	max	95.934	1	6.338	9	87.862	1	0	1	.042	5	.157	2
436			min	-6.814	10	-24.977	2	-30.405	5	0	5	-.014	1	-.137	3
437		10	max	96.073	1	6.136	9	87.862	1	0	1	.035	4	.163	2
438			min	-6.698	10	-25.218	2	-30.163	5	0	5	0	2	-.133	3
439		11	max	96.213	1	5.935	9	87.862	1	0	1	.033	4	.168	2
440			min	-6.581	10	-25.46	2	-29.921	5	0	5	.002	10	-.13	3
441		12	max	96.352	1	5.733	9	87.862	1	0	1	.043	1	.174	2
442			min	-6.465	10	-25.702	2	-29.679	5	0	5	.004	10	-.127	3
443		13	max	96.492	1	5.532	9	87.862	1	0	1	.062	1	.179	2
444			min	-6.348	10	-25.944	2	-29.437	5	0	5	.005	12	-.123	3
445		14	max	96.632	1	5.33	9	87.862	1	0	1	.081	1	.185	2
446			min	-6.232	10	-26.186	2	-29.195	5	0	5	.006	12	-.12	3
447		15	max	96.771	1	5.129	9	87.862	1	0	1	.1	1	.191	2
448			min	-6.116	10	-26.428	2	-28.953	5	0	5	.002	15	-.116	3
449		16	max	95.871	2	95.906	2	88.543	1	0	10	.121	1	.195	2
450			min	-5.31	3	-164.197	3	-27.51	5	0	4	-.001	5	-.111	3
451		17	max	96.01	2	95.664	2	88.543	1	0	10	.14	1	.174	2
452			min	-5.205	3	-164.378	3	-27.268	5	0	4	-.007	5	-.076	3
453		18	max	-.161	15	373.141	2	93.428	1	0	2	.16	1	.095	2
454			min	-161.357	1	-155.41	3	-57.715	5	0	3	-.019	5	-.042	3
455		19	max	-.119	15	372.899	2	93.428	1	0	2	.18	1	.014	2
456			min	-161.217	1	-155.591	3	-57.473	5	0	3	-.032	5	-.008	3
457	M13	1	max	230.348	4	310.508	1	-4.657	15	.012	1	.184	1	0	1
458			min	9.752	10	-338.37	3	-161.435	1	-.011	3	.004	15	0	3
459		2	max	221.547	4	219.187	1	-3.125	15	.012	1	.057	1	.256	3
460			min	9.752	10	-238.77	3	-123.647	1	-.011	3	0	15	-.235	1
461		3	max	212.745	4	127.866	1	-1.594	15	.012	1	.002	3	.424	3
462			min	9.752	10	-139.171	3	-85.858	1	-.011	3	-.036	1	-.39	1
463		4	max	203.944	4	36.545	1	-.062	15	.012	1	-.002	12	.504	3
464			min	9.752	10	-39.572	3	-48.07	1	-.011	3	-.096	1	-.463	1
465		5	max	195.142	4	60.028	3	2.167	5	.012	1	-.002	15	.495	3
466			min	9.752	10	-54.776	1	-10.282	1	-.011	3	-.122	1	-.455	1
467		6	max	186.341	4	159.627	3	27.506	1	.012	1	0	15	.397	3
468			min	9.752	10	-146.097	1	.448	12	-.011	3	-.114	1	-.365	1
469		7	max	177.539	4	259.226	3	65.294	1	.012	1	.005	5	.211	3
470			min	9.752	10	-237.418	1	1.942	12	-.011	3	-.073	1	-.195	1
471		8	max	168.738	4	358.826	3	103.082	1	.012	1	.012	4	.057	1
472			min	9.752	10	-328.739	1	3.437	12	-.011	3	0	3	-.064	3
473		9	max	159.936	4	458.425	3	140.87	1	.012	1	.111	1	.39	1
474			min	9.752	10	-420.06	1	4.931	12	-.011	3	.004	12	-.427	3
475		10	max	151.135	4	558.025	3	178.658	1	.011	2	.253	1	.804	1
476			min	9.752	10	-511.381	1	6.426	12	-.012	1	.009	12	-.879	3
477		11	max	112.625	4	420.059	1	-.747	15	.011	3	.106	1	.39	1
478			min	4.51	12	-458.425	3	-140.092	1	-.012	1	-.016	5	-.427	3
479		12	max	103.823	4	328.738	1	.924	5	.011	3	.001	2	.057	1
480			min	4.51	12	-358.826	3	-102.304	1	-.012	1	-.017	4	-.064	3
481		13	max	95.022	4	237.417	1	3.294	5	.011	3	-.005	12	.211	3
482			min	4.51	12	-259.226	3	-64.516	1	-.012	1	-.076	1	-.195	1
483		14	max	93.565	1	146.096	1	5.663	5	.011	3	-.006	12	.397	3
484			min	4.51	12	-159.627	3	-26.728	1	-.012	1	-.117	1	-.365	1



Company : Schletter, Inc.
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Job Number :
Model Name : Standard PVMini Racking System

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Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
485		15	max	93.565	1	54.775	1	11.06	1	.011	3	-.003	15	.495	3
486			min	4.51	12	-60.028	3	.754	10	-.012	1	-.124	1	-.455	1
487		16	max	93.565	1	39.572	3	48.848	1	.011	3	.004	5	.504	3
488			min	4.51	12	-36.546	1	2.694	12	-.012	1	-.097	1	-.463	1
489		17	max	93.565	1	139.171	3	86.636	1	.011	3	.014	5	.424	3
490			min	4.51	12	-127.867	1	4.189	12	-.012	1	-.037	1	-.39	1
491		18	max	93.565	1	238.771	3	124.424	1	.011	3	.057	1	.256	3
492			min	4.51	12	-219.188	1	5.683	12	-.012	1	.004	12	-.235	1
493		19	max	93.565	1	338.37	3	162.212	1	.011	3	.184	1	0	1
494			min	4.51	12	-310.509	1	7.178	12	-.012	1	.01	12	0	3
495	M16	1	max	57.475	5	373.192	2	-.119	15	.008	3	.18	1	0	2
496			min	-93.028	1	-155.624	3	-161.234	1	-.014	2	-.032	5	0	3
497		2	max	48.674	5	263.456	2	1.802	5	.008	3	.054	1	.118	3
498			min	-93.028	1	-110.015	3	-123.446	1	-.014	2	-.031	5	-.283	2
499		3	max	39.873	5	153.721	2	4.171	5	.008	3	-.001	12	.196	3
500			min	-93.028	1	-64.406	3	-85.658	1	-.014	2	-.039	1	-.468	2
501		4	max	31.071	5	43.986	2	6.541	5	.008	3	-.004	12	.233	3
502			min	-93.028	1	-18.797	3	-47.87	1	-.014	2	-.098	1	-.556	2
503		5	max	22.27	5	26.812	3	8.91	5	.008	3	-.005	12	.229	3
504			min	-93.028	1	-65.75	2	-10.082	1	-.014	2	-.124	1	-.547	2
505		6	max	13.468	5	72.421	3	27.706	1	.008	3	-.005	15	.185	3
506			min	-93.028	1	-175.485	2	.66	12	-.014	2	-.116	1	-.439	2
507		7	max	4.667	5	118.03	3	65.494	1	.008	3	.003	5	.1	3
508			min	-93.028	1	-285.22	2	2.155	12	-.014	2	-.075	1	-.235	2
509		8	max	-2.241	12	163.639	3	103.282	1	.008	3	.016	4	.068	2
510			min	-93.028	1	-394.955	2	3.649	12	-.014	2	-.003	3	-.025	3
511		9	max	-2.241	12	209.248	3	141.07	1	.008	3	.109	1	.468	2
512			min	-93.028	1	-504.691	2	5.144	12	-.014	2	.002	12	-.191	3
513		10	max	31.752	5	-14.854	15	178.858	1	.004	14	.251	1	.965	2
514			min	-95.802	1	-614.426	2	-10.221	3	-.014	2	.009	12	-.397	3
515		11	max	22.951	5	504.691	2	-.745	15	.014	2	.109	1	.468	2
516			min	-95.802	1	-209.248	3	-140.738	1	-.008	3	-.014	5	-.191	3
517		12	max	14.149	5	394.955	2	.923	5	.014	2	.001	2	.068	2
518			min	-95.802	1	-163.639	3	-102.95	1	-.008	3	-.015	4	-.025	3
519		13	max	5.348	5	285.22	2	3.293	5	.014	2	-.003	12	.1	3
520			min	-95.802	1	-118.03	3	-65.162	1	-.008	3	-.074	1	-.235	2
521		14	max	-2.165	15	175.485	2	5.662	5	.014	2	-.004	12	.185	3
522			min	-95.802	1	-72.421	3	-27.374	1	-.008	3	-.115	1	-.439	2
523		15	max	-4.766	12	65.749	2	10.538	4	.014	2	-.001	15	.229	3
524			min	-95.802	1	-26.812	3	.644	12	-.008	3	-.123	1	-.547	2
525		16	max	-4.766	12	18.797	3	48.202	1	.014	2	.006	5	.233	3
526			min	-95.802	1	-43.986	2	2.139	12	-.008	3	-.097	1	-.556	2
527		17	max	-4.766	12	64.406	3	85.99	1	.014	2	.016	5	.196	3
528			min	-95.802	1	-153.721	2	3.633	12	-.008	3	-.037	1	-.468	2
529		18	max	-4.766	12	110.015	3	123.778	1	.014	2	.056	1	.118	3
530			min	-95.802	1	-263.456	2	5.128	12	-.008	3	.003	12	-.283	2
531		19	max	-4.766	12	155.624	3	161.566	1	.014	2	.183	1	0	2
532			min	-95.802	1	-373.192	2	6.622	12	-.008	3	.008	12	0	5
533	M15	1	max	0	2	2.177	1	.03	3	0	1	0	1	0	1
534			min	-46.125	3	0	2	-.029	1	0	3	0	3	0	1
535		2	max	0	2	1.935	1	.03	3	0	1	0	1	0	2
536			min	-46.195	3	0	2	-.029	1	0	3	0	3	0	1
537		3	max	0	2	1.693	1	.03	3	0	1	0	1	0	2
538			min	-46.266	3	0	2	-.029	1	0	3	0	3	-.002	1
539		4	max	0	2	1.451	1	.03	3	0	1	0	1	0	2
540			min	-46.336	3	0	2	-.029	1	0	3	0	3	-.003	1
541		5	max	0	2	1.209	1	.03	3	0	1	0	1	0	2

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
542			min	-46.407	3	0	2	-0.029	1	0	3	0	3	-0.003	1
543		6	max	0	2	.967	1	.03	3	0	1	0	1	0	2
544			min	-46.477	3	0	2	-0.029	1	0	3	0	3	-0.004	1
545		7	max	0	2	.726	1	.03	3	0	1	0	3	0	2
546			min	-46.548	3	0	2	-0.029	1	0	3	0	1	-0.004	1
547		8	max	0	2	.484	1	.03	3	0	1	0	3	0	2
548			min	-46.618	3	0	2	-0.029	1	0	3	0	1	-0.004	1
549		9	max	0	2	.242	1	.03	3	0	1	0	3	0	2
550			min	-46.689	3	0	2	-0.029	1	0	3	0	1	-0.005	1
551		10	max	0	2	0	1	.03	3	0	1	0	3	0	2
552			min	-46.759	3	0	1	-0.029	1	0	3	0	1	-0.005	1
553		11	max	0	2	0	2	.03	3	0	1	0	3	0	2
554			min	-46.83	3	-.242	1	-0.029	1	0	3	0	1	-0.005	1
555		12	max	0	2	0	2	.03	3	0	1	0	3	0	2
556			min	-46.9	3	-.484	1	-0.029	1	0	3	0	1	-0.004	1
557		13	max	0	2	0	2	.03	3	0	1	0	3	0	2
558			min	-46.971	3	-.726	1	-0.029	1	0	3	0	1	-0.004	1
559		14	max	0	2	0	2	.03	3	0	1	0	3	0	2
560			min	-47.041	3	-.967	1	-0.029	1	0	3	0	1	-0.004	1
561		15	max	0	2	0	2	.03	3	0	1	0	3	0	2
562			min	-47.112	3	-1.209	1	-0.029	1	0	3	0	1	-0.003	1
563		16	max	0	2	0	2	.03	3	0	1	0	3	0	2
564			min	-47.182	3	-1.451	1	-0.029	1	0	3	0	1	-0.003	1
565		17	max	0	2	0	2	.03	3	0	1	0	3	0	2
566			min	-47.253	3	-1.693	1	-0.029	1	0	3	0	1	-0.002	1
567		18	max	0	2	0	2	.03	3	0	1	0	3	0	2
568			min	-47.323	3	-1.935	1	-0.029	1	0	3	0	1	0	1
569		19	max	0	2	0	2	.03	3	0	1	0	3	0	1
570			min	-47.394	3	-2.177	1	-0.029	1	0	3	0	1	0	1
571	M16A	1	max	-1.035	10	3.521	4	.277	4	0	3	0	3	0	1
572			min	-265.693	4	1.088	12	-.013	3	0	2	0	4	0	1
573		2	max	-.957	10	3.13	4	.25	4	0	3	0	3	0	12
574			min	-265.781	4	.967	12	-.013	3	0	2	0	4	-0.002	4
575		3	max	-.878	10	2.738	4	.222	4	0	3	0	3	0	12
576			min	-265.868	4	.846	12	-.013	3	0	2	0	4	-0.003	4
577		4	max	-.8	10	2.347	4	.194	4	0	3	0	3	-0.001	12
578			min	-265.955	4	.726	12	-.013	3	0	2	0	4	-0.004	4
579		5	max	-.722	10	1.956	4	.167	4	0	3	0	3	-0.002	12
580			min	-266.043	4	.605	12	-.013	3	0	2	0	1	-0.005	4
581		6	max	-.643	10	1.565	4	.139	4	0	3	0	5	-0.002	12
582			min	-266.13	4	.484	12	-.013	3	0	2	0	1	-0.006	4
583		7	max	-.565	10	1.174	4	.111	4	0	3	0	5	-0.002	12
584			min	-266.218	4	.363	12	-.013	3	0	2	0	1	-0.007	4
585		8	max	-.487	10	.782	4	.083	4	0	3	0	5	-0.002	12
586			min	-266.305	4	.242	12	-.013	3	0	2	0	1	-0.007	4
587		9	max	-.408	10	.391	4	.056	4	0	3	0	5	-0.002	12
588			min	-266.392	4	.121	12	-.013	3	0	2	0	1	-0.008	4
589		10	max	-.33	10	0	1	.028	4	0	3	0	5	-0.002	12
590			min	-266.48	4	0	1	-.013	3	0	2	0	1	-0.008	4
591		11	max	-.252	10	-.121	12	.021	1	0	3	0	5	-0.002	12
592			min	-266.567	4	-.391	4	-.013	3	0	2	0	1	-0.008	4
593		12	max	-.173	10	-.242	12	.021	1	0	3	0	5	-0.002	12
594			min	-266.654	4	-.782	4	-.031	5	0	2	0	1	-0.007	4
595		13	max	-.095	10	-.363	12	.021	1	0	3	0	5	-0.002	12
596			min	-266.742	4	-1.174	4	-.059	5	0	2	0	3	-0.007	4
597		14	max	-.017	10	-.484	12	.021	1	0	3	0	4	-0.002	12
598			min	-266.829	4	-1.565	4	-.086	5	0	2	0	3	-0.006	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
43		3	max	0	3	0	2	.012	5	2.303e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	-.001	1	-1.159e-3	5	NC	1	8599.187	14
45		4	max	0	3	0	2	.017	5	2.665e-4	1	NC	1	NC	1
46			min	0	2	-.003	3	-.001	1	-1.172e-3	5	NC	1	5606.982	14
47		5	max	0	3	0	2	.023	5	3.027e-4	1	NC	1	NC	1
48			min	0	2	-.004	3	-.001	1	-1.185e-3	5	NC	1	4124.296	14
49		6	max	0	3	0	2	.029	5	3.389e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	-1.198e-3	5	NC	1	3243.955	14
51		7	max	0	3	0	2	.035	4	3.752e-4	1	NC	1	NC	1
52			min	0	2	-.005	3	-.001	1	-1.211e-3	5	NC	1	2663.778	14
53		8	max	0	3	.001	2	.041	4	4.114e-4	1	NC	1	NC	1
54			min	0	2	-.006	3	0	1	-1.224e-3	5	NC	1	2254.395	14
55		9	max	0	3	.002	2	.047	4	4.476e-4	1	NC	1	NC	1
56			min	0	2	-.006	3	0	2	-1.237e-3	5	NC	1	1951.217	14
57		10	max	0	3	.002	2	.052	4	4.838e-4	1	NC	1	NC	1
58			min	0	2	-.007	3	0	10	-1.25e-3	5	NC	1	1718.426	14
59		11	max	.001	3	.003	2	.058	4	5.2e-4	1	NC	1	NC	1
60			min	0	2	-.007	3	0	10	-1.263e-3	5	NC	1	1534.582	14
61		12	max	.001	3	.003	2	.064	4	5.563e-4	1	NC	1	NC	1
62			min	-.001	2	-.008	3	0	12	-1.276e-3	5	NC	1	1386.07	14
63		13	max	.001	3	.004	2	.069	4	5.925e-4	1	NC	1	NC	1
64			min	-.001	2	-.008	3	0	12	-1.289e-3	5	NC	1	1263.843	14
65		14	max	.001	3	.005	2	.075	4	6.287e-4	1	NC	1	NC	1
66			min	-.001	2	-.008	3	0	12	-1.302e-3	5	9415.819	2	1161.65	14
67		15	max	.002	3	.006	2	.08	4	6.649e-4	1	NC	1	NC	2
68			min	-.001	2	-.009	3	0	12	-1.315e-3	5	7942.04	2	1075.044	14
69		16	max	.002	3	.007	2	.086	4	7.011e-4	1	NC	1	NC	2
70			min	-.001	2	-.009	3	0	12	-1.328e-3	5	6796.151	2	1000.768	14
71		17	max	.002	3	.008	2	.091	4	7.374e-4	1	NC	3	NC	2
72			min	-.001	2	-.009	3	0	12	-1.341e-3	5	5895.903	2	936.388	14
73		18	max	.002	3	.009	2	.097	4	7.736e-4	1	NC	3	NC	2
74			min	-.002	2	-.009	3	0	12	-1.354e-3	5	5182.303	2	880.047	14
75		19	max	.002	3	.01	2	.102	4	8.098e-4	1	NC	3	NC	2
76			min	-.002	2	-.009	3	0	12	-1.366e-3	5	4612.692	2	830.303	14
77	M4	1	max	.002	1	.012	2	0	12	7.088e-3	5	NC	1	NC	3
78			min	0	12	-.01	3	-.108	4	-1.176e-3	1	NC	1	179.483	4
79		2	max	.002	1	.011	2	0	12	7.088e-3	5	NC	1	NC	3
80			min	0	12	-.009	3	-.099	4	-1.176e-3	1	NC	1	195.663	4
81		3	max	.002	1	.01	2	0	12	7.088e-3	5	NC	1	NC	3
82			min	0	12	-.009	3	-.09	4	-1.176e-3	1	NC	1	214.922	4
83		4	max	.002	1	.01	2	0	12	7.088e-3	5	NC	1	NC	3
84			min	0	12	-.008	3	-.081	4	-1.176e-3	1	NC	1	238.07	4
85		5	max	.002	1	.009	2	0	12	7.088e-3	5	NC	1	NC	2
86			min	0	12	-.008	3	-.073	4	-1.176e-3	1	NC	1	266.213	4
87		6	max	.002	1	.008	2	0	12	7.088e-3	5	NC	1	NC	2
88			min	0	12	-.007	3	-.064	4	-1.176e-3	1	NC	1	300.887	4
89		7	max	.001	1	.008	2	0	12	7.088e-3	5	NC	1	NC	2
90			min	0	12	-.006	3	-.056	4	-1.176e-3	1	NC	1	344.279	4
91		8	max	.001	1	.007	2	0	12	7.088e-3	5	NC	1	NC	2
92			min	0	12	-.006	3	-.048	4	-1.176e-3	1	NC	1	399.595	4
93		9	max	.001	1	.006	2	0	12	7.088e-3	5	NC	1	NC	2
94			min	0	12	-.005	3	-.041	4	-1.176e-3	1	NC	1	471.68	4
95		10	max	.001	1	.006	2	0	12	7.088e-3	5	NC	1	NC	2
96			min	0	12	-.005	3	-.034	4	-1.176e-3	1	NC	1	568.152	4
97		11	max	0	1	.005	2	0	12	7.088e-3	5	NC	1	NC	1
98			min	0	12	-.004	3	-.028	4	-1.176e-3	1	NC	1	701.558	4
99		12	max	0	1	.005	2	0	12	7.088e-3	5	NC	1	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
157		3	max	0	3	.003	2	.012	5	2.119e-5	1	NC	1	NC	1
158			min	0	2	-.004	3	0	2	-1.184e-3	4	NC	1	NC	1
159		4	max	.001	3	.004	2	.018	5	2.161e-5	1	NC	1	NC	1
160			min	-.001	2	-.006	3	0	2	-1.182e-3	4	NC	1	NC	1
161		5	max	.001	3	.006	2	.024	5	2.203e-5	1	NC	1	NC	1
162			min	-.002	2	-.008	3	0	2	-1.179e-3	4	7970.879	2	NC	1
163		6	max	.002	3	.007	2	.03	5	2.246e-5	1	NC	3	NC	1
164			min	-.002	2	-.009	3	0	2	-1.177e-3	4	6391.958	2	NC	1
165		7	max	.002	3	.009	2	.036	5	3.241e-5	3	NC	3	NC	1
166			min	-.002	2	-.011	3	0	1	-1.174e-3	4	5314.15	2	NC	1
167		8	max	.002	3	.01	2	.042	5	4.693e-5	3	NC	3	NC	1
168			min	-.003	2	-.013	3	0	1	-1.172e-3	4	4525.316	2	NC	1
169		9	max	.003	3	.012	2	.048	5	6.145e-5	3	NC	3	NC	1
170			min	-.003	2	-.014	3	-.001	1	-1.169e-3	4	3919.725	2	NC	1
171		10	max	.003	3	.013	2	.054	5	7.598e-5	3	NC	3	NC	1
172			min	-.004	2	-.016	3	-.001	1	-1.167e-3	4	3438.772	2	NC	1
173		11	max	.003	3	.015	2	.06	4	9.05e-5	3	NC	3	NC	1
174			min	-.004	2	-.017	3	-.001	1	-1.164e-3	4	3047.288	2	NC	1
175		12	max	.004	3	.017	2	.065	4	1.05e-4	3	NC	3	NC	1
176			min	-.004	2	-.018	3	-.002	1	-1.162e-3	4	2722.793	2	NC	1
177		13	max	.004	3	.019	2	.071	4	1.195e-4	3	NC	3	NC	1
178			min	-.005	2	-.02	3	-.002	1	-1.159e-3	4	2450.16	2	NC	1
179		14	max	.004	3	.021	2	.076	4	1.341e-4	3	NC	3	NC	1
180			min	-.005	2	-.021	3	-.002	1	-1.157e-3	4	2218.754	2	NC	1
181		15	max	.005	3	.023	2	.081	4	1.486e-4	3	NC	3	NC	1
182			min	-.006	2	-.022	3	-.002	1	-1.155e-3	4	2020.822	2	NC	1
183		16	max	.005	3	.025	2	.087	4	1.631e-4	3	NC	3	NC	1
184			min	-.006	2	-.023	3	-.002	1	-1.152e-3	4	1850.536	2	NC	1
185		17	max	.005	3	.027	2	.092	4	1.776e-4	3	NC	3	NC	1
186			min	-.006	2	-.023	3	-.002	1	-1.15e-3	4	1703.408	2	NC	1
187		18	max	.006	3	.029	2	.097	4	1.922e-4	3	NC	3	NC	1
188			min	-.007	2	-.024	3	-.003	1	-1.147e-3	4	1575.906	2	NC	1
189		19	max	.006	3	.031	2	.102	4	2.067e-4	3	NC	3	NC	1
190			min	-.007	2	-.025	3	-.003	1	-1.145e-3	4	1465.215	2	NC	1
191	M8	1	max	.006	1	.037	2	.003	1	6.882e-3	4	NC	1	NC	2
192			min	0	3	-.028	3	-.108	4	-2.085e-4	1	NC	1	179.676	4
193		2	max	.006	1	.035	2	.003	1	6.882e-3	4	NC	1	NC	2
194			min	0	3	-.027	3	-.099	4	-2.085e-4	1	NC	1	195.873	4
195		3	max	.005	1	.033	2	.002	1	6.882e-3	4	NC	1	NC	2
196			min	0	3	-.025	3	-.09	4	-2.085e-4	1	NC	1	215.152	4
197		4	max	.005	1	.031	2	.002	1	6.882e-3	4	NC	1	NC	2
198			min	0	3	-.024	3	-.081	4	-2.085e-4	1	NC	1	238.324	4
199		5	max	.005	1	.029	2	.002	1	6.882e-3	4	NC	1	NC	2
200			min	0	3	-.022	3	-.073	4	-2.085e-4	1	NC	1	266.497	4
201		6	max	.004	1	.027	2	.002	1	6.882e-3	4	NC	1	NC	1
202			min	0	3	-.02	3	-.064	4	-2.085e-4	1	NC	1	301.206	4
203		7	max	.004	1	.025	2	.002	1	6.882e-3	4	NC	1	NC	1
204			min	0	3	-.019	3	-.056	4	-2.085e-4	1	NC	1	344.644	4
205		8	max	.004	1	.023	2	.001	1	6.882e-3	4	NC	1	NC	1
206			min	0	3	-.017	3	-.048	4	-2.085e-4	1	NC	1	400.017	4
207		9	max	.003	1	.021	2	.001	1	6.882e-3	4	NC	1	NC	1
208			min	0	3	-.016	3	-.041	4	-2.085e-4	1	NC	1	472.176	4
209		10	max	.003	1	.019	2	0	1	6.882e-3	4	NC	1	NC	1
210			min	0	3	-.014	3	-.034	4	-2.085e-4	1	NC	1	568.749	4
211		11	max	.003	1	.017	2	0	1	6.882e-3	4	NC	1	NC	1
212			min	0	3	-.013	3	-.028	4	-2.085e-4	1	NC	1	702.294	4
213		12	max	.002	1	.015	2	0	1	6.882e-3	4	NC	1	NC	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
214			min	0	3	-.011	3	-.022	4	-2.085e-4	1	NC	1	894.721	4
215		13	max	.002	1	.012	2	0	1	6.882e-3	4	NC	1	NC	1
216			min	0	3	-.009	3	-.016	4	-2.085e-4	1	NC	1	1187.126	4
217		14	max	.002	1	.01	2	0	1	6.882e-3	4	NC	1	NC	1
218			min	0	3	-.008	3	-.012	4	-2.085e-4	1	NC	1	1664.281	4
219		15	max	.001	1	.008	2	0	1	6.882e-3	4	NC	1	NC	1
220			min	0	3	-.006	3	-.008	4	-2.085e-4	1	NC	1	2525.342	4
221		16	max	0	1	.006	2	0	1	6.882e-3	4	NC	1	NC	1
222			min	0	3	-.005	3	-.004	4	-2.085e-4	1	NC	1	4336.478	4
223		17	max	0	1	.004	2	0	1	6.882e-3	4	NC	1	NC	1
224			min	0	3	-.003	3	-.002	4	-2.085e-4	1	NC	1	9296.575	4
225		18	max	0	1	.002	2	0	1	6.882e-3	4	NC	1	NC	1
226			min	0	3	-.002	3	0	4	-2.085e-4	1	NC	1	NC	1
227		19	max	0	1	0	1	0	1	6.882e-3	4	NC	1	NC	1
228			min	0	1	0	1	0	1	-2.085e-4	1	NC	1	NC	1
229	M10	1	max	.003	1	.01	2	0	3	1.231e-3	1	NC	3	NC	1
230			min	-.003	3	-.01	3	-.009	4	-2.538e-4	3	3953.986	2	NC	1
231		2	max	.003	1	.009	2	0	3	1.167e-3	1	NC	3	NC	1
232			min	-.003	3	-.009	3	-.009	4	-2.459e-4	3	4306.163	2	NC	1
233		3	max	.003	1	.008	2	0	3	1.102e-3	1	NC	3	NC	1
234			min	-.003	3	-.009	3	-.009	4	-2.381e-4	3	4723.252	2	NC	1
235		4	max	.003	1	.008	2	0	3	1.037e-3	1	NC	3	NC	1
236			min	-.003	3	-.009	3	-.009	4	-2.302e-4	3	5220.473	2	NC	1
237		5	max	.003	1	.007	2	0	3	9.837e-4	14	NC	1	NC	1
238			min	-.003	3	-.008	3	-.009	4	-2.223e-4	3	5817.972	2	NC	1
239		6	max	.002	1	.006	2	0	3	1.048e-3	4	NC	1	NC	1
240			min	-.002	3	-.008	3	-.009	4	-2.144e-4	3	6542.846	2	NC	1
241		7	max	.002	1	.005	2	0	3	1.119e-3	4	NC	1	NC	1
242			min	-.002	3	-.007	3	-.009	4	-2.066e-4	3	7432.214	2	NC	1
243		8	max	.002	1	.005	2	0	3	1.191e-3	4	NC	1	NC	1
244			min	-.002	3	-.007	3	-.009	4	-1.987e-4	3	8538.032	2	NC	1
245		9	max	.002	1	.004	2	0	3	1.262e-3	4	NC	1	NC	1
246			min	-.002	3	-.006	3	-.008	4	-1.908e-4	3	9934.883	2	NC	1
247		10	max	.002	1	.003	2	0	3	1.333e-3	4	NC	1	NC	1
248			min	-.002	3	-.006	3	-.008	4	-1.83e-4	3	NC	1	NC	1
249		11	max	.001	1	.003	2	0	3	1.404e-3	4	NC	1	NC	1
250			min	-.001	3	-.005	3	-.008	4	-1.751e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	1.475e-3	4	NC	1	NC	1
252			min	-.001	3	-.005	3	-.007	4	-1.672e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	1.547e-3	4	NC	1	NC	1
254			min	-.001	3	-.004	3	-.006	4	-1.594e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	1.618e-3	4	NC	1	NC	1
256			min	0	3	-.004	3	-.006	4	-1.515e-4	3	NC	1	NC	1
257		15	max	0	1	.001	2	0	3	1.689e-3	4	NC	1	NC	1
258			min	0	3	-.003	3	-.005	4	-1.436e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	1.76e-3	4	NC	1	NC	1
260			min	0	3	-.002	3	-.004	4	-1.358e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.831e-3	4	NC	1	NC	1
262			min	0	3	-.002	3	-.003	4	-1.279e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.903e-3	4	NC	1	NC	1
264			min	0	3	0	3	-.001	4	-1.2e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	1.974e-3	4	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.122e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	5.286e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-9.319e-4	4	NC	1	NC	1
269		2	max	0	3	0	2	.005	4	3.588e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-1.042e-3	4	NC	1	9840.551	5

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
271		3	max	0	3	0	2	.01	4	1.889e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.152e-3	4	NC	1	4873.791	5
273		4	max	0	3	0	2	.014	4	1.906e-6	3	NC	1	NC	1
274			min	0	2	-.003	3	0	3	-1.262e-3	4	NC	1	3223.658	5
275		5	max	0	3	0	2	.019	4	-1.023e-5	12	NC	1	NC	1
276			min	0	2	-.004	3	-.001	1	-1.372e-3	4	NC	1	2402.136	5
277		6	max	0	3	0	2	.024	5	-2.103e-5	12	NC	1	NC	1
278			min	0	2	-.004	3	-.002	1	-1.482e-3	4	NC	1	1911.666	5
279		7	max	0	3	0	2	.029	5	-3.182e-5	12	NC	1	NC	1
280			min	0	2	-.005	3	-.003	1	-1.592e-3	4	NC	1	1586.423	5
281		8	max	0	3	.001	2	.034	5	-4.261e-5	12	NC	1	NC	1
282			min	0	2	-.006	3	-.004	1	-1.701e-3	4	NC	1	1355.357	5
283		9	max	0	3	.002	2	.039	5	-5.328e-5	10	NC	1	NC	2
284			min	0	2	-.007	3	-.005	1	-1.811e-3	4	NC	1	1182.955	5
285		10	max	0	3	.002	2	.044	5	-5.907e-5	10	NC	1	NC	2
286			min	0	2	-.007	3	-.006	1	-1.921e-3	4	NC	1	1049.494	5
287		11	max	.001	3	.003	2	.049	5	-6.485e-5	10	NC	1	NC	2
288			min	0	2	-.008	3	-.007	1	-2.031e-3	4	NC	1	943.145	5
289		12	max	.001	3	.003	2	.054	5	-7.064e-5	10	NC	1	NC	2
290			min	-.001	2	-.008	3	-.009	1	-2.141e-3	4	NC	1	856.385	5
291		13	max	.001	3	.004	2	.059	5	-7.642e-5	10	NC	1	NC	2
292			min	-.001	2	-.008	3	-.01	1	-2.251e-3	4	NC	1	784.197	5
293		14	max	.001	3	.005	2	.064	5	-8.221e-5	10	NC	1	NC	2
294			min	-.001	2	-.008	3	-.011	1	-2.361e-3	4	9426.766	2	723.113	5
295		15	max	.002	3	.006	2	.069	5	-8.799e-5	10	NC	1	NC	3
296			min	-.001	2	-.009	3	-.013	1	-2.471e-3	4	7950.496	2	670.653	5
297		16	max	.002	3	.007	2	.074	5	-9.378e-5	10	NC	1	NC	3
298			min	-.001	2	-.009	3	-.014	1	-2.581e-3	4	6802.835	2	625.001	5
299		17	max	.002	3	.008	2	.079	5	-9.957e-5	10	NC	3	NC	3
300			min	-.001	2	-.009	3	-.016	1	-2.691e-3	4	5901.306	2	584.797	5
301		18	max	.002	3	.009	2	.084	5	-1.054e-4	10	NC	3	NC	3
302			min	-.002	2	-.009	3	-.017	1	-2.801e-3	4	5186.766	2	549	5
303		19	max	.002	3	.01	2	.089	5	-1.111e-4	10	NC	3	NC	3
304			min	-.002	2	-.009	3	-.018	1	-2.911e-3	4	4616.457	2	516.807	5
305	M12	1	max	.002	1	.012	2	.015	1	8.499e-3	4	NC	1	NC	3
306			min	0	12	-.01	3	-.098	5	1.091e-4	10	NC	1	196.542	5
307		2	max	.002	1	.011	2	.014	1	8.499e-3	4	NC	1	NC	3
308			min	0	12	-.009	3	-.09	5	1.091e-4	10	NC	1	214.256	5
309		3	max	.002	1	.01	2	.013	1	8.499e-3	4	NC	1	NC	3
310			min	0	12	-.009	3	-.082	5	1.091e-4	10	NC	1	235.339	5
311		4	max	.002	1	.01	2	.011	1	8.499e-3	4	NC	1	NC	3
312			min	0	12	-.008	3	-.074	5	1.091e-4	10	NC	1	260.681	5
313		5	max	.002	1	.009	2	.01	1	8.499e-3	4	NC	1	NC	3
314			min	0	12	-.008	3	-.066	5	1.091e-4	10	NC	1	291.49	5
315		6	max	.002	1	.008	2	.009	1	8.499e-3	4	NC	1	NC	3
316			min	0	12	-.007	3	-.059	5	1.091e-4	10	NC	1	329.448	5
317		7	max	.001	1	.008	2	.008	1	8.499e-3	4	NC	1	NC	3
318			min	0	12	-.007	3	-.051	5	1.091e-4	10	NC	1	376.951	5
319		8	max	.001	1	.007	2	.007	1	8.499e-3	4	NC	1	NC	3
320			min	0	12	-.006	3	-.044	5	1.091e-4	10	NC	1	437.505	5
321		9	max	.001	1	.006	2	.006	1	8.499e-3	4	NC	1	NC	3
322			min	0	12	-.005	3	-.037	5	1.091e-4	10	NC	1	516.415	5
323		10	max	.001	1	.006	2	.005	1	8.499e-3	4	NC	1	NC	2
324			min	0	12	-.005	3	-.031	5	1.091e-4	10	NC	1	622.02	5
325		11	max	0	1	.005	2	.004	1	8.499e-3	4	NC	1	NC	2
326			min	0	12	-.004	3	-.025	5	1.091e-4	10	NC	1	768.054	5
327		12	max	0	1	.005	2	.003	1	8.499e-3	4	NC	1	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
328		min	0	12	-.004	3	-.02	5	1.091e-4	10	NC	1	978.475	5
329		max	0	1	.004	2	.002	1	8.499e-3	4	NC	1	NC	2
330		min	0	12	-.003	3	-.015	5	1.091e-4	10	NC	1	1298.219	5
331		max	0	1	.003	2	.002	1	8.499e-3	4	NC	1	NC	1
332		min	0	12	-.003	3	-.011	5	1.091e-4	10	NC	1	1819.978	5
333		max	0	1	.003	2	.001	1	8.499e-3	4	NC	1	NC	1
334		min	0	12	-.002	3	-.007	5	1.091e-4	10	NC	1	2761.519	5
335		max	0	1	.002	2	0	1	8.499e-3	4	NC	1	NC	1
336		min	0	12	-.002	3	-.004	5	1.091e-4	10	NC	1	4741.907	5
337		max	0	1	.001	2	0	1	8.499e-3	4	NC	1	NC	1
338		min	0	12	-.001	3	-.002	5	1.091e-4	10	NC	1	NC	1
339		max	0	1	0	2	0	1	8.499e-3	4	NC	1	NC	1
340		min	0	12	0	3	0	5	1.091e-4	10	NC	1	NC	1
341		max	0	1	0	1	0	1	8.499e-3	4	NC	1	NC	1
342		min	0	1	0	1	0	1	1.091e-4	10	NC	1	NC	1
343	M1	max	.009	3	.026	3	.011	5	1.389e-2	1	NC	1	NC	1
344		min	-.009	2	-.025	1	-.006	1	-1.504e-2	3	NC	1	NC	1
345		max	.009	3	.015	3	.015	5	6.44e-3	1	NC	4	NC	2
346		min	-.009	2	-.015	2	-.013	1	-7.457e-3	3	4173.375	1	6495.872	1
347		max	.009	3	.005	3	.019	5	6.914e-4	5	NC	4	NC	2
348		min	-.009	2	-.004	2	-.018	1	-8.738e-4	1	2153.482	2	3938.948	1
349		max	.009	3	.005	1	.024	5	7.088e-4	5	NC	5	NC	3
350		min	-.009	2	-.003	3	-.02	1	-7.473e-4	1	1508.55	2	3259.339	1
351		max	.009	3	.012	1	.03	5	7.263e-4	5	NC	5	NC	3
352		min	-.009	2	-.01	3	-.02	1	-6.209e-4	1	1196.507	2	2472.14	5
353		max	.009	3	.019	2	.036	5	7.437e-4	5	NC	5	NC	3
354		min	-.009	2	-.016	3	-.019	1	-4.945e-4	1	1018.385	2	1893.416	5
355		max	.009	3	.024	2	.042	5	7.611e-4	5	NC	5	NC	2
356		min	-.009	2	-.02	3	-.017	1	-3.681e-4	1	908.728	2	1519.761	5
357		max	.009	3	.028	2	.048	5	7.785e-4	5	NC	5	NC	2
358		min	-.009	2	-.023	3	-.014	1	-2.417e-4	1	840.19	2	1261.479	5
359		max	.009	3	.03	2	.054	5	7.96e-4	5	NC	5	NC	2
360		min	-.009	2	-.025	3	-.01	1	-1.152e-4	1	799.87	2	1072.005	4
361		max	.009	3	.031	2	.061	5	8.134e-4	5	NC	5	NC	1
362		min	-.009	2	-.025	3	-.006	1	-4.657e-6	2	781.633	2	912.551	4
363		max	.009	3	.031	2	.068	4	8.584e-4	4	NC	5	NC	1
364		min	-.009	2	-.024	3	-.001	1	1.79e-5	10	783.301	2	793.789	4
365		max	.009	3	.028	2	.075	4	9.052e-4	4	NC	5	NC	2
366		min	-.009	2	-.022	3	0	10	2.96e-5	12	805.832	2	703.268	4
367		max	.009	3	.025	2	.083	4	9.521e-4	4	NC	5	NC	2
368		min	-.009	2	-.019	3	0	12	3.399e-5	12	853.825	2	633.092	4
369		max	.008	3	.019	2	.09	4	9.989e-4	4	NC	5	NC	3
370		min	-.009	2	-.015	3	0	12	3.839e-5	12	937.831	2	578.065	4
371		max	.008	3	.012	2	.096	4	1.046e-3	4	NC	5	NC	3
372		min	-.009	2	-.009	3	0	12	4.279e-5	12	1080.945	2	534.67	4
373		max	.008	3	.004	1	.102	4	1.453e-3	4	NC	4	NC	3
374		min	-.009	2	-.003	3	0	12	4.588e-5	12	1339.477	2	500.474	4
375		max	.008	3	.005	3	.108	4	1.045e-2	4	NC	4	NC	2
376		min	-.009	2	-.007	2	0	12	-1.059e-4	1	1892.241	2	473.809	4
377		max	.008	3	.013	3	.112	4	8.252e-3	2	NC	4	NC	2
378		min	-.009	2	-.019	2	0	10	-3.526e-3	3	3655.521	2	453.452	4
379		max	.008	3	.022	3	.115	4	1.672e-2	2	NC	1	NC	1
380		min	-.009	2	-.032	2	-.004	1	-7.146e-3	3	NC	1	439.107	4
381	M5	max	.025	3	.077	3	.01	5	7.616e-6	4	NC	1	NC	1
382		min	-.03	2	-.079	1	-.007	1	5.612e-8	10	NC	1	NC	1
383		max	.025	3	.046	3	.015	5	3.516e-4	5	NC	5	NC	1
384		min	-.03	2	-.046	1	-.006	1	-6.592e-5	1	1389.906	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
385		3	max	.025	3	.017	3	.02	5	6.9e-4	5	NC	5	NC	1
386			min	-.03	2	-.014	1	-.005	1	-1.315e-4	1	714.589	1	NC	1
387		4	max	.025	3	.012	1	.025	5	7.189e-4	5	NC	5	NC	1
388			min	-.03	2	-.008	3	-.004	1	-1.245e-4	1	502.994	1	NC	1
389		5	max	.025	3	.036	1	.031	5	7.478e-4	5	NC	5	NC	1
390			min	-.03	2	-.028	3	-.004	1	-1.176e-4	1	401.076	1	NC	1
391		6	max	.025	3	.055	1	.037	5	7.766e-4	5	NC	15	NC	1
392			min	-.03	2	-.044	3	-.003	1	-1.107e-4	1	343.241	1	NC	1
393		7	max	.025	3	.07	2	.044	5	8.055e-4	5	NC	15	NC	1
394			min	-.03	2	-.057	3	-.003	1	-1.038e-4	1	306.701	2	NC	1
395		8	max	.025	3	.081	2	.051	5	8.344e-4	5	NC	15	NC	1
396			min	-.03	2	-.065	3	-.003	1	-9.687e-5	1	283.053	2	NC	1
397		9	max	.025	3	.089	2	.058	5	8.633e-4	5	NC	15	NC	1
398			min	-.03	2	-.07	3	-.003	1	-8.996e-5	1	269.014	2	NC	1
399		10	max	.025	3	.092	2	.064	5	8.922e-4	5	NC	15	NC	1
400			min	-.03	2	-.071	3	-.002	1	-8.304e-5	1	262.472	2	NC	1
401		11	max	.025	3	.09	2	.071	5	9.211e-4	5	NC	15	NC	1
402			min	-.03	2	-.069	3	-.002	1	-7.612e-5	1	262.663	2	NC	1
403		12	max	.025	3	.084	2	.078	5	9.5e-4	5	NC	15	NC	1
404			min	-.03	2	-.063	3	-.002	1	-6.921e-5	1	269.885	2	NC	1
405		13	max	.025	3	.073	2	.085	5	9.789e-4	5	NC	15	NC	1
406			min	-.03	2	-.054	3	-.002	1	-6.229e-5	1	285.668	2	NC	1
407		14	max	.025	3	.058	2	.091	5	1.008e-3	5	NC	15	NC	1
408			min	-.03	2	-.042	3	-.002	1	-5.538e-5	1	313.547	2	9398.595	4
409		15	max	.025	3	.037	2	.097	4	1.037e-3	5	NC	5	NC	1
410			min	-.03	2	-.026	3	-.003	1	-4.915e-5	2	361.303	2	9280.619	4
411		16	max	.024	3	.011	1	.102	4	1.425e-3	5	NC	5	NC	1
412			min	-.03	2	-.008	3	-.003	1	-5.153e-5	1	448.013	2	NC	1
413		17	max	.024	3	.014	3	.107	4	1.042e-2	4	NC	5	NC	1
414			min	-.03	2	-.022	2	-.003	1	-2.924e-4	1	635.21	2	NC	1
415		18	max	.024	3	.037	3	.112	4	5.346e-3	4	NC	5	NC	1
416			min	-.03	2	-.059	2	-.003	1	-1.499e-4	1	1233.563	2	NC	1
417		19	max	.024	3	.061	3	.116	4	2.078e-6	5	NC	1	NC	1
418			min	-.03	2	-.098	2	-.003	1	-2.297e-7	3	NC	1	NC	1
419	M9	1	max	.009	3	.026	3	.008	5	1.504e-2	3	NC	1	NC	1
420			min	-.009	2	-.026	1	-.008	1	-1.389e-2	1	NC	1	NC	1
421		2	max	.009	3	.015	3	.008	5	7.44e-3	3	NC	4	NC	2
422			min	-.009	2	-.015	1	-.002	1	-6.733e-3	1	4174.218	1	7608.705	1
423		3	max	.009	3	.005	3	.009	4	2.907e-4	1	NC	4	NC	2
424			min	-.009	2	-.004	2	0	3	-2.353e-5	3	2154.327	2	4734.305	1
425		4	max	.009	3	.005	1	.011	4	1.834e-4	1	NC	4	NC	3
426			min	-.009	2	-.003	3	0	3	-3.153e-5	3	1509.154	2	4020.604	1
427		5	max	.009	3	.012	2	.014	4	8.681e-5	4	NC	5	NC	3
428			min	-.009	2	-.01	3	-.001	3	-3.953e-5	3	1196.984	2	3997.459	1
429		6	max	.009	3	.019	2	.018	4	8.42e-5	5	NC	5	NC	3
430			min	-.009	2	-.016	3	-.002	3	-4.753e-5	3	1018.784	2	3869.124	14
431		7	max	.009	3	.024	2	.022	4	1.036e-4	5	NC	5	NC	2
432			min	-.009	2	-.02	3	-.002	3	-1.386e-4	1	909.078	2	2942.615	4
433		8	max	.009	3	.028	2	.027	4	1.231e-4	5	NC	5	NC	2
434			min	-.009	2	-.023	3	-.003	3	-2.459e-4	1	840.506	2	2233.174	4
435		9	max	.009	3	.03	2	.033	5	1.425e-4	5	NC	5	NC	1
436			min	-.009	2	-.025	3	-.005	1	-3.532e-4	1	800.163	2	1757.8	4
437		10	max	.009	3	.031	2	.04	5	1.619e-4	5	NC	5	NC	1
438			min	-.009	2	-.025	3	-.008	1	-4.605e-4	1	781.912	2	1423.784	4
439		11	max	.009	3	.03	2	.047	5	1.814e-4	5	NC	5	NC	2
440			min	-.009	2	-.024	3	-.012	1	-5.679e-4	1	783.573	2	1180.063	4
441		12	max	.009	3	.028	2	.055	5	2.008e-4	5	NC	5	NC	2



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
499	3	max	.003	1	.156	3	.142	1	7.21e-3	2	NC	5	NC	3
500		min	-.116	4	-.346	2	.009	10	-4.626e-3	3	612.416	2	1305.838	1
501	4	max	.003	1	.195	3	.212	1	8.297e-3	2	NC	5	NC	12
502		min	-.116	4	-.435	2	.015	10	-5.282e-3	3	476.49	2	882.732	1
503	5	max	.003	1	.208	3	.246	1	9.384e-3	2	NC	5	NC	12
504		min	-.116	4	-.463	2	.018	10	-5.938e-3	3	445.849	2	762.658	1
505	6	max	.003	1	.196	3	.235	1	1.047e-2	2	NC	5	NC	12
506		min	-.116	4	-.43	2	.015	10	-6.594e-3	3	482.803	2	799.955	1
507	7	max	.003	1	.164	3	.18	1	1.156e-2	2	NC	5	NC	5
508		min	-.116	4	-.349	2	.007	10	-7.25e-3	3	607.321	2	1036.386	1
509	8	max	.003	1	.12	3	.098	1	1.264e-2	2	NC	5	NC	5
510		min	-.116	4	-.242	2	-.003	10	-7.906e-3	3	918.227	2	1860.942	1
511	9	max	.003	1	.08	3	.028	3	1.373e-2	2	NC	4	NC	2
512		min	-.116	4	-.143	2	-.015	2	-8.562e-3	3	1735.975	2	8664.082	1
513	10	max	.003	1	.061	3	.024	3	1.482e-2	2	NC	4	NC	1
514		min	-.116	4	-.098	2	-.03	2	-9.218e-3	3	2922.754	2	9176.41	2
515	11	max	.003	1	.08	3	.025	3	1.373e-2	2	NC	4	NC	2
516		min	-.116	4	-.143	2	-.014	2	-8.562e-3	3	1735.975	2	9331.17	1
517	12	max	.003	1	.12	3	.095	1	1.265e-2	2	NC	5	NC	3
518		min	-.116	4	-.242	2	-.003	10	-7.905e-3	3	918.227	2	1917.32	1
519	13	max	.004	1	.164	3	.176	1	1.156e-2	2	NC	5	NC	5
520		min	-.116	4	-.349	2	.007	10	-7.248e-3	3	607.321	2	1060.64	1
521	14	max	.004	1	.196	3	.23	1	1.047e-2	2	NC	5	NC	5
522		min	-.116	4	-.43	2	.007	15	-6.591e-3	3	482.803	2	817.254	1
523	15	max	.004	1	.208	3	.241	1	9.385e-3	2	NC	5	NC	5
524		min	-.116	4	-.463	2	.002	15	-5.934e-3	3	445.849	2	779.545	1
525	16	max	.004	1	.195	3	.207	1	8.299e-3	2	NC	5	NC	3
526		min	-.116	4	-.435	2	-.005	5	-5.277e-3	3	476.49	2	904.542	1
527	17	max	.004	1	.156	3	.137	1	7.212e-3	2	NC	5	NC	3
528		min	-.115	4	-.346	2	-.011	5	-4.621e-3	3	612.416	2	1345.556	1
529	18	max	.004	1	.095	3	.055	1	6.125e-3	2	NC	5	NC	3
530		min	-.115	4	-.205	2	-.011	5	-3.964e-3	3	1114.758	2	3190.945	1
531	19	max	.004	1	.022	3	.008	3	5.039e-3	2	NC	1	NC	1
532		min	-.115	4	-.032	2	-.009	2	-3.307e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.76e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-6.58e-4	5	NC	1	NC	1
535	2	max	0	3	-.001	15	.014	4	8.745e-4	3	NC	5	NC	1
536		min	-.001	5	-.02	1	0	3	-6.89e-4	5	5266.127	1	7393.462	4
537	3	max	0	3	-.002	15	.031	4	1.373e-3	3	NC	5	NC	1
538		min	-.002	5	-.039	1	-.003	3	-1.178e-3	1	2679.753	1	3315.933	4
539	4	max	0	3	-.004	15	.049	4	1.872e-3	3	NC	5	NC	9
540		min	-.003	5	-.057	1	-.007	3	-1.753e-3	1	1838.467	1	2092.018	4
541	5	max	0	3	-.005	15	.067	4	2.37e-3	3	NC	5	NC	9
542		min	-.004	5	-.073	1	-.012	3	-2.328e-3	1	1434.574	1	1545.976	4
543	6	max	0	3	-.005	15	.082	4	2.869e-3	3	NC	15	9775.611	9
544		min	-.005	5	-.087	1	-.017	3	-2.902e-3	1	1207.346	1	1260.083	4
545	7	max	0	3	-.006	15	.094	4	3.367e-3	3	NC	15	7705.91	9
546		min	-.006	5	-.098	1	-.022	3	-3.477e-3	1	1070.698	1	1102.481	4
547	8	max	0	3	-.006	15	.101	4	3.866e-3	3	NC	15	6394.514	9
548		min	-.007	5	-.106	1	-.027	3	-4.052e-3	1	988.689	1	1021.262	4
549	9	max	0	3	-.006	15	.104	4	4.365e-3	3	NC	15	5532.064	9
550		min	-.008	5	-.111	1	-.032	3	-4.626e-3	1	944.546	1	994.783	4
551	10	max	0	3	-.006	15	.102	4	4.863e-3	3	NC	15	4961.795	9
552		min	-.009	5	-.113	1	-.036	3	-5.201e-3	1	930.582	1	1016.554	4
553	11	max	0	3	-.006	15	.095	4	5.362e-3	3	NC	15	4601.101	9
554		min	-.01	5	-.111	1	-.038	3	-5.776e-3	1	944.546	1	1091.335	4
555	12	max	0	3	-.005	15	.083	4	5.86e-3	3	NC	15	4409.352	9



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
 Maximum concrete compression stress (psi): 0
 Resultant tension force (lb): 405
 Resultant compression force (lb): 0
 Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
 Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
 Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
 Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)

Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status
Steel	733	6071	0.12	Pass
Concrete breakout	1465	7233	0.20	Pass (Governs)
Adhesive	1465	8418	0.17	Pass
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status
Steel	500	3156	0.16	Pass
T Concrete breakout x+	999	4043	0.25	Pass (Governs)
Concrete breakout y-	999	11720	0.09	Pass (Governs)
Pryout	999	15580	0.06	Pass
Interaction check	$N_{ua} / \phi N_n$	$V_{ua} / \phi V_n$	Combined Ratio	Permissible Status

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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.