

Schletter, Inc.	Standard PVMini Racking System Representative Calculations - ASCE 7-05	20° Tilt w/o Seismic Design
HCV		

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-05 - Chapter 6, Wind Loads
- ASCE 7-05 - Chapter 7, Snow Loads
- ASCE 7-05 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2003, 2006, 2009
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-05, Eq. 7-2)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	90 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 12.72 psf Including the gust factor, $G=0.85$. (ASCE 7-05, Eq. 6-15)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.8W \\
 &1.2D + 1.6W + 0.5S \\
 &0.9D + 1.6W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (ASCE 7, Eq 2.3.2-1 through 2.3.2-7) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 1.0W \\
 &1.0D + 0.75L + 0.75W + 0.75S \\
 &0.6D + 1.0W^M \quad (ASCE 7, Eq 2.4.1-1 through 2.4.1-8) \text{ \& } (ASCE 7, Section 12.4.3.2) \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

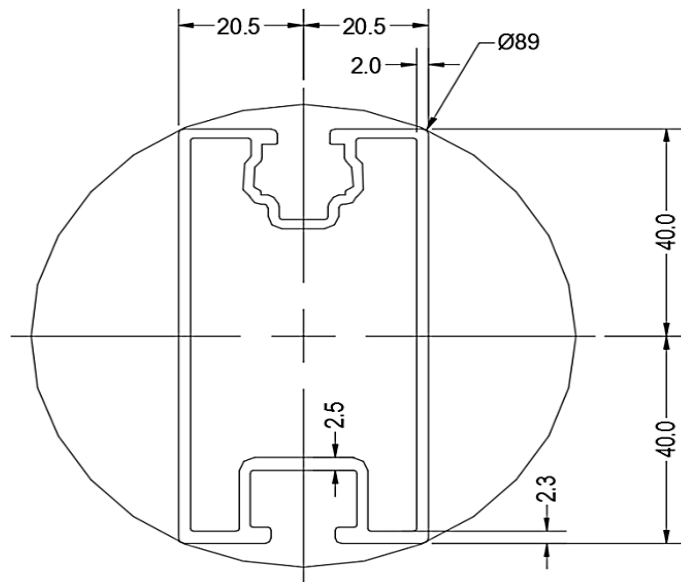
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

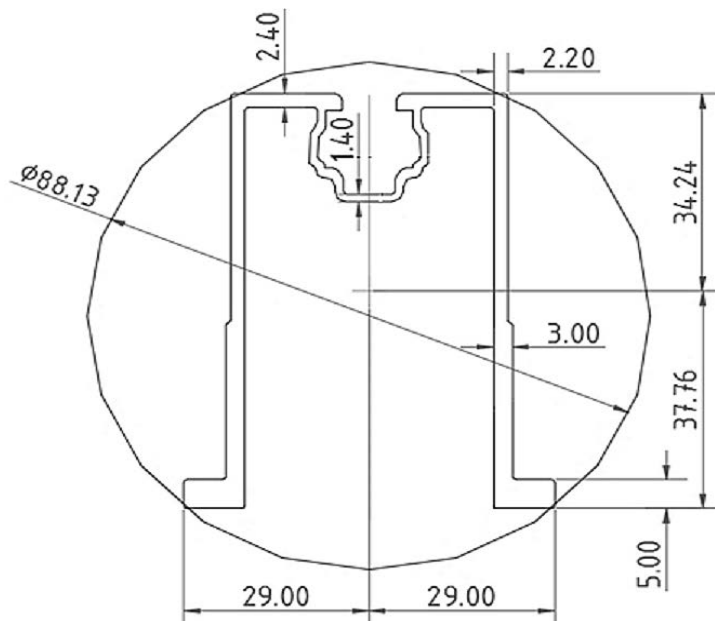
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	93 in
ΦF_{ty} STRONG-AXIS =	28.83 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.058 k-ft
M_z =	0.197 k-ft
$M_{y \text{ allowable}}$ =	1.791 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	83%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.72 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.565 k-ft
M_z =	0.000 k-ft
P_n =	0.226 k
$M_{y \text{ allowable}}$ =	1.458 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	41%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.502 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	12%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.322 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	8%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.220 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.035 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	13%



A cross brace kit is required every 14 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

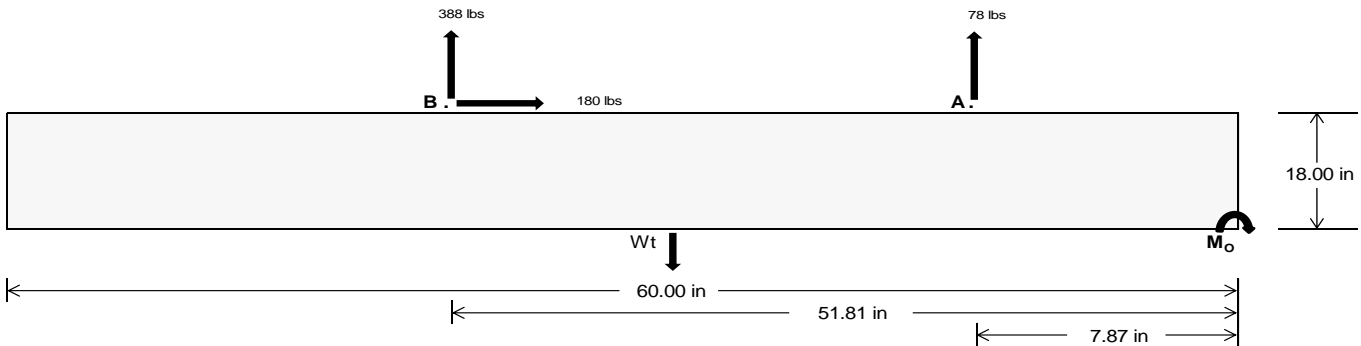
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	330.39	1617.06	k
Compressive Load =	1953.20	1514.63	k
Lateral Load =	4.40	749.41	k
Moment (Weak Axis) =	0.01	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC tables 1804.2 (2003, 2006) & 1806.2 (2009).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23949.2$ in-lbs
Resisting Force Required = 798.31 lbs
S.F. = 1.67
Weight Required = 1330.51 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 180.07 lbs
Friction = 0.4
Weight Required = 450.19 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 180.07 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 1.0W				1.0D + 0.75L + 0.75W + 0.75S				0.6D + 1.0W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	740 lbs	740 lbs	740 lbs	740 lbs	564 lbs	564 lbs	564 lbs	564 lbs	923 lbs	923 lbs	923 lbs	923 lbs	-156 lbs	-156 lbs	-156 lbs	-156 lbs
F_B	541 lbs	541 lbs	541 lbs	541 lbs	497 lbs	497 lbs	497 lbs	497 lbs	737 lbs	737 lbs	737 lbs	737 lbs	-776 lbs	-776 lbs	-776 lbs	-776 lbs
F_V	65 lbs	65 lbs	65 lbs	65 lbs	324 lbs	324 lbs	324 lbs	324 lbs	288 lbs	288 lbs	288 lbs	288 lbs	-360 lbs	-360 lbs	-360 lbs	-360 lbs
P_{total}	3275 lbs	3366 lbs	3456 lbs	3547 lbs	3055 lbs	3146 lbs	3236 lbs	3327 lbs	3654 lbs	3745 lbs	3835 lbs	3926 lbs	264 lbs	319 lbs	373 lbs	428 lbs
M	479 lbs-ft	479 lbs-ft	479 lbs-ft	479 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	622 lbs-ft	793 lbs-ft	793 lbs-ft	793 lbs-ft	793 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft	582 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.20 ft	0.20 ft	0.19 ft	0.19 ft	0.22 ft	0.21 ft	0.21 ft	0.21 ft	0.20 ft	2.20 ft	1.82 ft	1.56 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	294.6 psf	291.3 psf	288.2 psf	285.4 psf	251.8 psf	250.3 psf	249.0 psf	247.7 psf	294.8 psf	291.5 psf	288.4 psf	285.5 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	419.9 psf	411.1 psf	403.1 psf	395.6 psf	414.7 psf	406.2 psf	398.3 psf	391.1 psf	502.4 psf	490.1 psf	478.7 psf	468.3 psf	319.7 psf	164.1 psf	132.1 psf	120.0 psf

Maximum Bearing Pressure = 502 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

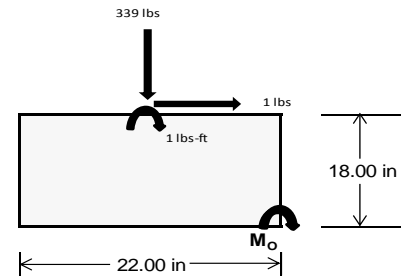
Overturning Check

$M_o = 308.0 \text{ ft-lbs}$
 Resisting Force Required = 336.05 lbs
 S.F. = 1.67
 Weight Required = 560.08 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	87 lbs	239 lbs	82 lbs	343 lbs	1044 lbs	339 lbs	25 lbs	70 lbs	24 lbs
F_v	4 lbs	3 lbs	0 lbs	18 lbs	17 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2555 lbs	2707 lbs	2551 lbs	2693 lbs	3393 lbs	2688 lbs	747 lbs	792 lbs	746 lbs
M	6 lbs-ft	5 lbs-ft	0 lbs-ft	31 lbs-ft	25 lbs-ft	2 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.81 ft	1.82 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	276.8 sqft	293.5 sqft	278.2 sqft	282.8 sqft	361.1 sqft	292.4 sqft	80.9 sqft	85.8 sqft	81.3 sqft
f_{max}	280.7 psf	297.2 psf	278.4 psf	304.8 psf	379.3 psf	294.1 psf	82.1 psf	86.9 psf	81.4 psf



Maximum Bearing Pressure = 379 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.399 k
Allowable Uplift =	1.214 k
Utilization =	<u>33%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.085 k
Allowable Uplift =	1.116 k
Utilization =	<u>97%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.502 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>26%</u>

Diagonal Strut

Maximum Axial Load =	0.322 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.220 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>21%</u>

Bracing

Maximum Axial Load =	0.035 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.047 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$193.965$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$210.771$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.791 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi_y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi_y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.29 \\
 &21.6567 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.29 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.458 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ ds &= 6.05 \\ rs &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} F_{cy}}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^* \lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{\max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy) / (\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

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Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	135.109	2	320.095	1	0	5	0	2	0	1	0	1
2		min	-180.871	3	-379.209	3	-.128	1	0	3	0	1	0	1
3	N7	max	0	15	525.352	1	-.058	15	0	15	0	1	0	1
4		min	-.181	1	-68.877	3	-1.499	1	-.003	1	0	1	0	1
5	N15	max	0	15	1502.459	1	.579	1	.001	1	0	1	0	1
6		min	-1.89	1	-254.145	3	-.278	3	0	3	0	1	0	1
7	N16	max	543.546	2	1165.1	1	-.179	10	0	1	0	1	0	1
8		min	-576.467	3	-1243.895	3	-32.526	3	0	3	0	1	0	1
9	N23	max	0	15	525.121	1	3.387	1	.006	1	0	1	0	1
10		min	-.181	1	-68.445	3	.123	15	0	15	0	1	0	1
11	N24	max	135.578	2	325.535	1	32.763	3	.002	1	0	1	0	1
12		min	-180.944	3	-376.438	3	.032	10	0	3	0	1	0	1
13	Totals:	max	812.237	2	4363.663	1	0	1						
14		min	-938.578	3	-2391.009	3	0	3						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	372.406	1	.644	4	.728	1	0	15	0	3	0	1
2			min	-359.197	3	.152	15	-.044	3	-.001	1	0	1	0	1
3		2	max	372.512	1	.602	4	.728	1	0	15	0	1	0	15
4			min	-359.117	3	.143	15	-.044	3	-.001	1	0	10	0	4
5		3	max	372.619	1	.561	4	.728	1	0	15	0	1	0	15
6			min	-359.037	3	.133	15	-.044	3	-.001	1	0	3	0	4
7		4	max	372.725	1	.52	4	.728	1	0	15	0	1	0	15
8			min	-358.957	3	.123	15	-.044	3	-.001	1	0	3	0	4
9		5	max	372.832	1	.478	4	.728	1	0	15	0	1	0	15
10			min	-358.877	3	.113	15	-.044	3	-.001	1	0	3	0	4
11		6	max	372.938	1	.437	4	.728	1	0	15	0	1	0	15
12			min	-358.797	3	.104	15	-.044	3	-.001	1	0	3	0	4
13		7	max	373.045	1	.396	4	.728	1	0	15	0	1	0	15
14			min	-358.717	3	.094	15	-.044	3	-.001	1	0	3	0	4
15		8	max	373.151	1	.355	4	.728	1	0	15	0	1	0	15
16			min	-358.637	3	.084	15	-.044	3	-.001	1	0	3	0	4
17		9	max	373.258	1	.313	4	.728	1	0	15	0	1	0	15
18			min	-358.557	3	.075	15	-.044	3	-.001	1	0	3	0	4
19		10	max	373.364	1	.272	4	.728	1	0	15	0	1	0	15
20			min	-358.477	3	.065	15	-.044	3	-.001	1	0	3	0	4
21		11	max	373.471	1	.231	4	.728	1	0	15	.001	1	0	15
22			min	-358.398	3	.055	15	-.044	3	-.001	1	0	3	0	4
23		12	max	373.578	1	.19	4	.728	1	0	15	.001	1	0	15
24			min	-358.318	3	.046	15	-.044	3	-.001	1	0	3	0	4
25		13	max	373.684	1	.148	4	.728	1	0	15	.001	1	0	15
26			min	-358.238	3	.036	15	-.044	3	-.001	1	0	3	0	4
27		14	max	373.791	1	.107	4	.728	1	0	15	.001	1	0	15
28			min	-358.158	3	.026	15	-.044	3	-.001	1	0	3	0	4
29		15	max	373.897	1	.072	2	.728	1	0	15	.001	1	0	15
30			min	-358.078	3	.014	12	-.044	3	-.001	1	0	3	0	4
31		16	max	374.004	1	.04	2	.728	1	0	15	.002	1	0	15
32			min	-357.998	3	-.005	3	-.044	3	-.001	1	0	3	0	4
33		17	max	374.11	1	.01	10	.728	1	0	15	.002	1	0	15
34			min	-357.918	3	-.03	1	-.044	3	-.001	1	0	3	0	4
35		18	max	374.217	1	-.013	15	.728	1	0	15	.002	1	0	15
36			min	-357.838	3	-.062	1	-.044	3	-.001	1	0	3	0	4
37		19	max	374.323	1	-.022	15	.728	1	0	15	.002	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-357.758	3	-.099	4	-.044	3	-.001	1	0	3	0	4
39	M3	1	max	63.878	2	1.795	4	-.023	15	0	.002	1	0	4
40		min	-98.261	9	.423	15	-.688	1	0	1	0	15	0	15
41		2	max	63.81	2	1.618	4	-.023	15	0	.002	1	0	4
42		min	-98.317	9	.381	15	-.688	1	0	1	0	15	0	15
43		3	max	63.742	2	1.44	4	-.023	15	0	.002	1	0	2
44		min	-98.374	9	.339	15	-.688	1	0	1	0	15	0	3
45		4	max	63.674	2	1.262	4	-.023	15	0	.002	1	0	15
46		min	-98.43	9	.297	15	-.688	1	0	1	0	15	0	4
47		5	max	63.606	2	1.085	4	-.023	15	0	.001	1	0	15
48		min	-98.487	9	.256	15	-.688	1	0	1	0	15	0	4
49		6	max	63.538	2	.907	4	-.023	15	0	.001	1	0	15
50		min	-98.543	9	.214	15	-.688	1	0	1	0	15	0	4
51		7	max	63.47	2	.729	4	-.023	15	0	.001	1	0	15
52		min	-98.6	9	.172	15	-.688	1	0	1	0	15	0	4
53		8	max	63.403	2	.552	4	-.023	15	0	.001	1	0	15
54		min	-98.657	9	.13	15	-.688	1	0	1	0	15	0	4
55		9	max	63.335	2	.374	4	-.023	15	0	0	1	0	15
56		min	-98.713	9	.089	15	-.688	1	0	1	0	15	-.001	4
57		10	max	63.267	2	.196	4	-.023	15	0	0	1	0	15
58		min	-98.77	9	.047	15	-.688	1	0	1	0	15	-.001	4
59		11	max	63.199	2	.03	2	-.023	15	0	0	1	0	15
60		min	-98.826	9	-.003	3	-.688	1	0	1	0	15	-.001	4
61		12	max	63.131	2	-.037	15	-.023	15	0	0	1	0	15
62		min	-98.883	9	-.159	4	-.688	1	0	1	0	15	-.001	4
63		13	max	63.063	2	-.078	15	-.023	15	0	0	1	0	15
64		min	-98.939	9	-.337	4	-.688	1	0	1	0	12	-.001	4
65		14	max	62.995	2	-.12	15	-.023	15	0	0	1	0	15
66		min	-98.996	9	-.514	4	-.688	1	0	1	0	12	-.001	4
67		15	max	62.927	2	-.162	15	-.023	15	0	0	1	0	15
68		min	-99.052	9	-.692	4	-.688	1	0	1	0	3	0	4
69		16	max	62.86	2	-.204	15	-.023	15	0	0	15	0	15
70		min	-99.109	9	-.869	4	-.688	1	0	1	0	1	0	4
71		17	max	62.792	2	-.245	15	-.023	15	0	0	15	0	15
72		min	-99.166	9	-1.047	4	-.688	1	0	1	0	1	0	4
73		18	max	62.724	2	-.287	15	-.023	15	0	0	15	0	15
74		min	-99.222	9	-1.225	4	-.688	1	0	1	0	1	0	4
75		19	max	62.656	2	-.329	15	-.023	15	0	0	15	0	1
76		min	-99.279	9	-1.402	4	-.688	1	0	1	0	1	0	1
77	M4	1	max	524.188	1	0	1	-.058	15	0	0	3	0	1
78		min	-69.751	3	0	1	-1.643	1	0	1	0	1	0	1
79		2	max	524.252	1	0	1	-.058	15	0	0	15	0	1
80		min	-69.702	3	0	1	-1.643	1	0	1	0	1	0	1
81		3	max	524.317	1	0	1	-.058	15	0	0	15	0	1
82		min	-69.654	3	0	1	-1.643	1	0	1	0	1	0	1
83		4	max	524.382	1	0	1	-.058	15	0	0	15	0	1
84		min	-69.605	3	0	1	-1.643	1	0	1	0	1	0	1
85		5	max	524.446	1	0	1	-.058	15	0	0	15	0	1
86		min	-69.557	3	0	1	-1.643	1	0	1	0	1	0	1
87		6	max	524.511	1	0	1	-.058	15	0	0	15	0	1
88		min	-69.508	3	0	1	-1.643	1	0	1	0	1	0	1
89		7	max	524.576	1	0	1	-.058	15	0	0	15	0	1
90		min	-69.46	3	0	1	-1.643	1	0	1	0	1	0	1
91		8	max	524.641	1	0	1	-.058	15	0	0	15	0	1
92		min	-69.411	3	0	1	-1.643	1	0	1	-.001	1	0	1
93		9	max	524.705	1	0	1	-.058	15	0	0	15	0	1
94		min	-69.363	3	0	1	-1.643	1	0	1	-.001	1	0	1



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	524.77	1	0	1	-.058	15	0	1	0	15	0	1
96		min	-69.314	3	0	1	-1.643	1	0	1	-.001	1	0	1
97	11	max	524.835	1	0	1	-.058	15	0	1	0	15	0	1
98		min	-69.266	3	0	1	-1.643	1	0	1	-.001	1	0	1
99	12	max	524.899	1	0	1	-.058	15	0	1	0	15	0	1
100		min	-69.217	3	0	1	-1.643	1	0	1	-.002	1	0	1
101	13	max	524.964	1	0	1	-.058	15	0	1	0	15	0	1
102		min	-69.169	3	0	1	-1.643	1	0	1	-.002	1	0	1
103	14	max	525.029	1	0	1	-.058	15	0	1	0	15	0	1
104		min	-69.12	3	0	1	-1.643	1	0	1	-.002	1	0	1
105	15	max	525.094	1	0	1	-.058	15	0	1	0	15	0	1
106		min	-69.071	3	0	1	-1.643	1	0	1	-.002	1	0	1
107	16	max	525.158	1	0	1	-.058	15	0	1	0	15	0	1
108		min	-69.023	3	0	1	-1.643	1	0	1	-.002	1	0	1
109	17	max	525.223	1	0	1	-.058	15	0	1	0	15	0	1
110		min	-68.974	3	0	1	-1.643	1	0	1	-.002	1	0	1
111	18	max	525.288	1	0	1	-.058	15	0	1	0	15	0	1
112		min	-68.926	3	0	1	-1.643	1	0	1	-.003	1	0	1
113	19	max	525.352	1	0	1	-.058	15	0	1	0	15	0	1
114		min	-68.877	3	0	1	-1.643	1	0	1	-.003	1	0	1
115	M6	1	max	1217.896	1	.638	.263	1	0	1	0	3	0	1
116		min	-1172.656	3	.151	15	-.125	3	0	15	0	1	0	1
117	2	max	1218.002	1	.596	4	.263	1	0	1	0	3	0	15
118		min	-1172.576	3	.142	15	-.125	3	0	15	0	11	0	4
119	3	max	1218.109	1	.555	4	.263	1	0	1	0	2	0	15
120		min	-1172.496	3	.132	15	-.125	3	0	15	0	15	0	4
121	4	max	1218.215	1	.514	4	.263	1	0	1	0	1	0	15
122		min	-1172.416	3	.122	15	-.125	3	0	15	0	12	0	4
123	5	max	1218.322	1	.473	4	.263	1	0	1	0	1	0	15
124		min	-1172.336	3	.113	15	-.125	3	0	15	0	3	0	4
125	6	max	1218.428	1	.431	4	.263	1	0	1	0	1	0	15
126		min	-1172.256	3	.103	15	-.125	3	0	15	0	3	0	4
127	7	max	1218.535	1	.39	4	.263	1	0	1	0	1	0	15
128		min	-1172.176	3	.093	15	-.125	3	0	15	0	3	0	4
129	8	max	1218.641	1	.349	4	.263	1	0	1	0	1	0	15
130		min	-1172.096	3	.084	15	-.125	3	0	15	0	3	0	4
131	9	max	1218.748	1	.314	2	.263	1	0	1	0	1	0	15
132		min	-1172.016	3	.074	15	-.125	3	0	15	0	3	0	4
133	10	max	1218.855	1	.282	2	.263	1	0	1	0	1	0	15
134		min	-1171.936	3	.064	15	-.125	3	0	15	0	3	0	4
135	11	max	1218.961	1	.25	2	.263	1	0	1	0	1	0	15
136		min	-1171.857	3	.051	12	-.125	3	0	15	0	3	0	4
137	12	max	1219.068	1	.218	2	.263	1	0	1	0	1	0	15
138		min	-1171.777	3	.035	12	-.125	3	0	15	0	3	0	4
139	13	max	1219.174	1	.185	2	.263	1	0	1	0	1	0	15
140		min	-1171.697	3	.019	12	-.125	3	0	15	0	3	0	4
141	14	max	1219.281	1	.153	2	.263	1	0	1	0	1	0	15
142		min	-1171.617	3	0	3	-.125	3	0	15	0	3	0	4
143	15	max	1219.387	1	.121	2	.263	1	0	1	0	1	0	15
144		min	-1171.537	3	-.023	3	-.125	3	0	15	0	3	0	4
145	16	max	1219.494	1	.089	2	.263	1	0	1	0	1	0	15
146		min	-1171.457	3	-.047	3	-.125	3	0	15	0	3	0	2
147	17	max	1219.6	1	.057	2	.263	1	0	1	0	1	0	15
148		min	-1171.377	3	-.071	3	-.125	3	0	15	0	3	0	2
149	18	max	1219.707	1	.025	2	.263	1	0	1	0	1	0	15
150		min	-1171.297	3	-.096	3	-.125	3	0	15	0	3	0	2
151	19	max	1219.813	1	-.007	2	.263	1	0	1	0	1	0	15



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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1171.217	3	-.12	3	-.125	3	0	15	0	3	0	2
153	M7	1	max	322.476	2	1.794	4	.014	1	0	2	0	2	2
154		min	-260.048	3	.423	15	-.003	10	0	3	0	3	0	12
155		2	max	322.408	2	1.617	4	.014	1	0	2	0	2	2
156		min	-260.099	3	.381	15	-.003	10	0	3	0	3	0	12
157		3	max	322.34	2	1.439	4	.014	1	0	2	0	2	2
158		min	-260.15	3	.339	15	-.003	10	0	3	0	3	0	3
159		4	max	322.272	2	1.261	4	.014	1	0	2	0	2	2
160		min	-260.2	3	.297	15	-.003	10	0	3	0	3	0	3
161		5	max	322.204	2	1.084	4	.014	1	0	2	0	2	15
162		min	-260.251	3	.256	15	-.003	10	0	3	0	3	0	4
163		6	max	322.137	2	.906	4	.014	1	0	2	0	2	15
164		min	-260.302	3	.214	15	-.003	10	0	3	0	3	0	4
165		7	max	322.069	2	.728	4	.014	1	0	2	0	2	15
166		min	-260.353	3	.172	15	-.003	10	0	3	0	3	0	4
167		8	max	322.001	2	.551	4	.014	1	0	2	0	2	15
168		min	-260.404	3	.13	15	-.003	10	0	3	0	3	-.001	4
169		9	max	321.933	2	.373	4	.014	1	0	2	0	2	15
170		min	-260.455	3	.089	15	-.003	10	0	3	0	3	-.001	4
171		10	max	321.865	2	.215	2	.014	1	0	2	0	2	15
172		min	-260.506	3	.042	12	-.003	10	0	3	0	3	-.001	4
173		11	max	321.797	2	.077	2	.014	1	0	2	0	2	15
174		min	-260.557	3	-.044	3	-.003	10	0	3	0	3	-.001	4
175		12	max	321.729	2	-.037	15	.014	1	0	2	0	2	15
176		min	-260.608	3	-.16	4	-.003	10	0	3	0	3	-.001	4
177		13	max	321.662	2	-.078	15	.014	1	0	2	0	2	15
178		min	-260.659	3	-.338	4	-.003	10	0	3	0	3	-.001	4
179		14	max	321.594	2	-.12	15	.014	1	0	2	0	2	15
180		min	-260.709	3	-.515	4	-.003	10	0	3	0	3	-.001	4
181		15	max	321.526	2	-.162	15	.014	1	0	2	0	2	15
182		min	-260.76	3	-.693	4	-.003	10	0	3	0	3	0	4
183		16	max	321.458	2	-.204	15	.014	1	0	2	0	2	15
184		min	-260.811	3	-.871	4	-.003	10	0	3	0	3	0	4
185		17	max	321.39	2	-.246	15	.014	1	0	2	0	2	15
186		min	-260.862	3	-1.048	4	-.003	10	0	3	0	3	0	4
187		18	max	321.322	2	-.287	15	.014	1	0	2	0	2	15
188		min	-260.913	3	-1.226	4	-.003	10	0	3	0	3	0	4
189		19	max	321.254	2	-.329	15	.014	1	0	2	0	2	1
190		min	-260.964	3	-1.404	4	-.003	10	0	3	0	3	0	1
191	M8	1	max	1501.295	1	0	1	.767	1	0	1	0	15	0
192		min	-255.019	3	0	1	-.267	3	0	1	0	1	0	1
193		2	max	1501.359	1	0	1	.767	1	0	1	0	1	0
194		min	-254.97	3	0	1	-.267	3	0	1	0	3	0	1
195		3	max	1501.424	1	0	1	.767	1	0	1	0	1	0
196		min	-254.922	3	0	1	-.267	3	0	1	0	3	0	1
197		4	max	1501.489	1	0	1	.767	1	0	1	0	1	0
198		min	-254.873	3	0	1	-.267	3	0	1	0	3	0	1
199		5	max	1501.554	1	0	1	.767	1	0	1	0	1	0
200		min	-254.825	3	0	1	-.267	3	0	1	0	3	0	1
201		6	max	1501.618	1	0	1	.767	1	0	1	0	1	0
202		min	-254.776	3	0	1	-.267	3	0	1	0	3	0	1
203		7	max	1501.683	1	0	1	.767	1	0	1	0	1	0
204		min	-254.728	3	0	1	-.267	3	0	1	0	3	0	1
205		8	max	1501.748	1	0	1	.767	1	0	1	0	1	0
206		min	-254.679	3	0	1	-.267	3	0	1	0	3	0	1
207		9	max	1501.812	1	0	1	.767	1	0	1	0	1	0
208		min	-254.631	3	0	1	-.267	3	0	1	0	3	0	1





Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266	M11	min	-344.576	3	-1.142	1	-.152	1	0	3	0	1	0	4
267		max	63.582	2	1.8	4	.805	1	.001	1	0	3	0	4
268		min	-98.158	9	.423	15	.015	12	0	15	-.002	1	0	15
269		2 max	63.514	2	1.622	4	.805	1	.001	1	0	3	0	4
270		min	-98.215	9	.382	15	.015	12	0	15	-.002	1	0	12
271		3 max	63.446	2	1.445	4	.805	1	.001	1	0	3	0	2
272		min	-98.271	9	.34	15	.015	12	0	15	-.002	1	0	3
273		4 max	63.378	2	1.267	4	.805	1	.001	1	0	3	0	15
274		min	-98.328	9	.298	15	.015	12	0	15	-.002	1	0	3
275		5 max	63.311	2	1.09	4	.805	1	.001	1	0	3	0	15
276		min	-98.384	9	.256	15	.015	12	0	15	-.001	1	0	4
277		6 max	63.243	2	.912	4	.805	1	.001	1	0	3	0	15
278		min	-98.441	9	.215	15	.015	12	0	15	-.001	1	0	4
279		7 max	63.175	2	.734	4	.805	1	.001	1	0	3	0	15
280		min	-98.497	9	.173	15	.015	12	0	15	-.001	1	0	4
281		8 max	63.107	2	.557	4	.805	1	.001	1	0	3	0	15
282		min	-98.554	9	.131	15	.015	12	0	15	0	1	0	4
283		9 max	63.039	2	.379	4	.805	1	.001	1	0	3	0	15
284		min	-98.61	9	.089	15	.015	12	0	15	0	1	-.001	4
285		10 max	62.971	2	.201	4	.805	1	.001	1	0	3	0	15
286		min	-98.667	9	.048	15	.015	12	0	15	0	1	-.001	4
287		11 max	62.903	2	.03	2	.805	1	.001	1	0	3	0	15
288		min	-98.724	9	-.02	3	.015	12	0	15	0	1	-.001	4
289		12 max	62.836	2	-.036	15	.805	1	.001	1	0	3	0	15
290		min	-98.78	9	-.154	4	.015	12	0	15	0	1	-.001	4
291		13 max	62.768	2	-.078	15	.805	1	.001	1	0	3	0	15
292		min	-98.837	9	-.332	4	.015	12	0	15	0	2	-.001	4
293		14 max	62.7	2	-.119	15	.805	1	.001	1	0	3	0	15
294		min	-98.893	9	-.509	4	.015	12	0	15	0	10	-.001	4
295		15 max	62.632	2	-.161	15	.805	1	.001	1	0	1	0	15
296		min	-98.95	9	-.687	4	.015	12	0	15	0	15	0	4
297		16 max	62.564	2	-.203	15	.805	1	.001	1	0	1	0	15
298		min	-99.006	9	-.865	4	.015	12	0	15	0	15	0	4
299		17 max	62.496	2	-.245	15	.805	1	.001	1	0	1	0	15
300		min	-99.063	9	-1.042	4	.015	12	0	15	0	15	0	4
301		18 max	62.428	2	-.286	15	.805	1	.001	1	0	1	0	15
302		min	-99.119	9	-1.22	4	.015	12	0	15	0	15	0	4
303		19 max	62.36	2	-.328	15	.805	1	.001	1	.001	1	0	1
304		min	-99.176	9	-1.398	4	.015	12	0	15	0	15	0	1
305		1 M12 max	523.956	1	0	1	3.708	1	0	1	0	1	0	1
306		min	-69.318	3	0	1	.123	15	0	1	0	3	0	1
307		2 max	524.021	1	0	1	3.708	1	0	1	0	1	0	1
308		min	-69.27	3	0	1	.123	15	0	1	0	15	0	1
309		3 max	524.086	1	0	1	3.708	1	0	1	0	1	0	1
310		min	-69.221	3	0	1	.123	15	0	1	0	15	0	1
311		4 max	524.15	1	0	1	3.708	1	0	1	.001	1	0	1
312		min	-69.173	3	0	1	.123	15	0	1	0	15	0	1
313		5 max	524.215	1	0	1	3.708	1	0	1	.001	1	0	1
314		min	-69.124	3	0	1	.123	15	0	1	0	15	0	1
315		6 max	524.28	1	0	1	3.708	1	0	1	.002	1	0	1
316		min	-69.075	3	0	1	.123	15	0	1	0	15	0	1
317		7 max	524.345	1	0	1	3.708	1	0	1	.002	1	0	1
318		min	-69.027	3	0	1	.123	15	0	1	0	15	0	1
319		8 max	524.409	1	0	1	3.708	1	0	1	.002	1	0	1
320		min	-68.978	3	0	1	.123	15	0	1	0	15	0	1
321		9 max	524.474	1	0	1	3.708	1	0	1	.003	1	0	1
322		min	-68.93	3	0	1	.123	15	0	1	0	15	0	1





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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-129.961	1	-152.643	3	-74.347	1	0	1	-.142	1	-.008	3
381	M5	max	287.94	1	1115.344	3	-.065	10	0	1	.004	1	.021	3
382		min	8.289	12	-1228.907	1	-29.239	3	0	3	0	10	-.028	1
383		max	288.036	1	1115.147	3	-.065	10	0	1	0	2	.239	1
384		min	8.337	12	-1229.169	1	-29.239	3	0	3	-.003	3	-.221	3
385		max	220.999	1	8.766	9	3.324	3	0	3	0	2	.501	1
386		min	5.467	10	-69.227	3	-.399	2	0	1	-.009	3	-.458	3
387		max	221.095	1	8.547	9	3.324	3	0	3	0	2	.505	1
388		min	5.546	10	-69.423	3	-.399	2	0	1	-.008	3	-.443	3
389		max	221.19	1	8.328	9	3.324	3	0	3	0	2	.509	1
390		min	5.626	10	-69.62	3	-.399	2	0	1	-.008	3	-.427	3
391		max	221.286	1	8.11	9	3.324	3	0	3	0	2	.514	1
392		min	5.705	10	-69.817	3	-.399	2	0	1	-.007	3	-.412	3
393		max	221.381	1	7.891	9	3.324	3	0	3	0	2	.518	1
394		min	5.785	10	-70.014	3	-.399	2	0	1	-.006	3	-.397	3
395		max	221.477	1	7.672	9	3.324	3	0	3	0	2	.523	1
396		min	5.865	10	-70.211	3	-.399	2	0	1	-.005	3	-.382	3
397		max	221.572	1	7.454	9	3.324	3	0	3	0	2	.527	1
398		min	5.944	10	-70.407	3	-.399	2	0	1	-.005	3	-.367	3
399		max	221.668	1	7.235	9	3.324	3	0	3	0	10	.532	1
400		min	6.024	10	-70.604	3	-.399	2	0	1	-.004	3	-.351	3
401		max	221.763	1	7.016	9	3.324	3	0	3	0	10	.537	1
402		min	6.103	10	-70.801	3	-.399	2	0	1	-.003	3	-.336	3
403		max	221.859	1	6.798	9	3.324	3	0	3	0	10	.542	1
404		min	6.183	10	-70.998	3	-.399	2	0	1	-.002	3	-.321	3
405		max	221.954	1	6.579	9	3.324	3	0	3	0	10	.547	1
406		min	6.263	10	-71.195	3	-.399	2	0	1	-.002	1	-.305	3
407		max	222.05	1	6.36	9	3.324	3	0	3	0	10	.552	1
408		min	6.342	10	-71.391	3	-.399	2	0	1	-.002	1	-.29	3
409		max	222.145	1	6.142	9	3.324	3	0	3	0	15	.556	1
410		min	6.422	10	-71.588	3	-.399	2	0	1	-.002	1	-.274	3
411		max	289.083	2	170.657	2	3.299	3	0	1	0	3	.561	1
412		min	-100.473	3	-259.717	3	-.409	2	0	15	-.001	1	-.257	3
413		max	289.178	2	170.394	2	3.299	3	0	1	0	3	.565	1
414		min	-100.402	3	-259.913	3	-.409	2	0	15	0	1	-.201	3
415		max	-8.927	12	1384.719	1	3.034	3	0	3	.002	3	.271	1
416		min	-288.723	1	-500.457	3	-.097	2	0	1	0	2	-.093	3
417		max	-8.879	12	1384.457	1	3.034	3	0	3	.002	3	.015	3
418		min	-288.628	1	-500.654	3	-.097	2	0	1	0	2	-.03	1
419	M9	max	129.861	1	338.014	3	94.33	1	0	3	-.005	15	.014	1
420		min	4.338	15	-372.296	1	3.323	15	0	1	-.142	1	-.011	3
421		max	129.956	1	337.817	3	94.33	1	0	3	-.003	12	.095	1
422		min	4.367	15	-372.558	1	3.323	15	0	1	-.122	1	-.084	3
423		max	110.236	1	6.999	9	68.022	1	0	1	.003	3	.174	1
424		min	4.151	15	-20.948	3	.979	12	0	15	-.1	1	-.156	3
425		max	110.332	1	6.78	9	68.022	1	0	1	.003	3	.174	1
426		min	4.18	15	-21.145	3	.979	12	0	15	-.085	1	-.151	3
427		max	110.427	1	6.561	9	68.022	1	0	1	.003	3	.174	1
428		min	4.209	15	-21.342	3	.979	12	0	15	-.07	1	-.146	3
429		max	110.523	1	6.343	9	68.022	1	0	1	.004	3	.174	1
430		min	4.238	15	-21.538	3	.979	12	0	15	-.056	1	-.142	3
431		max	110.618	1	6.124	9	68.022	1	0	1	.004	3	.175	1
432		min	4.267	15	-21.735	3	.979	12	0	15	-.041	1	-.137	3
433		max	110.714	1	5.905	9	68.022	1	0	1	.004	3	.175	1
434		min	4.296	15	-21.932	3	.979	12	0	15	-.026	1	-.132	3
435		max	110.809	1	5.687	9	68.022	1	0	1	.005	3	.175	1
436		min	4.324	15	-22.129	3	.979	12	0	15	-.011	1	-.128	3







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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	2	0	1	.028	3	0	1	0	3	0	2
552		min	-34.798	3	0	1	-.034	1	0	3	0	1	-.006	4
553	11	max	0	2	0	2	.028	3	0	1	0	3	0	2
554		min	-34.858	3	-.3	4	-.034	1	0	3	0	1	-.005	4
555	12	max	0	2	0	2	.028	3	0	1	0	3	0	2
556		min	-34.917	3	-.6	4	-.034	1	0	3	0	1	-.005	4
557	13	max	0	2	0	2	.028	3	0	1	0	3	0	2
558		min	-34.977	3	-.9	4	-.034	1	0	3	0	1	-.005	4
559	14	max	0	2	0	2	.028	3	0	1	0	3	0	2
560		min	-35.037	3	-1.2	4	-.034	1	0	3	0	1	-.004	4
561	15	max	0	2	0	2	.028	3	0	1	0	3	0	2
562		min	-35.096	3	-1.5	4	-.034	1	0	3	0	1	-.004	4
563	16	max	0	2	0	2	.028	3	0	1	0	3	0	2
564		min	-35.156	3	-1.8	4	-.034	1	0	3	0	1	-.003	4
565	17	max	0	2	0	2	.028	3	0	1	0	3	0	2
566		min	-35.216	3	-2.1	4	-.034	1	0	3	0	1	-.002	4
567	18	max	0	2	0	2	.028	3	0	1	0	3	0	2
568		min	-35.275	3	-2.4	4	-.034	1	0	3	0	1	-.001	4
569	19	max	0	2	0	2	.028	3	0	1	0	3	0	1
570		min	-35.335	3	-2.7	4	-.034	1	0	3	0	1	0	1
571	M16A	1	max	- .792	10	2.7	.022	1	0	3	0	3	0	1
572		min	-34.845	3	.635	15	-.012	3	0	1	0	1	0	1
573	2	max	-.726	10	2.4	4	.022	1	0	3	0	3	0	15
574		min	-34.785	3	.564	15	-.012	3	0	1	0	1	-.001	4
575	3	max	-.66	10	2.1	4	.022	1	0	3	0	3	0	15
576		min	-34.725	3	.494	15	-.012	3	0	1	0	1	-.002	4
577	4	max	-.594	10	1.8	4	.022	1	0	3	0	3	0	15
578		min	-34.666	3	.423	15	-.012	3	0	1	0	1	-.003	4
579	5	max	-.527	10	1.5	4	.022	1	0	3	0	3	0	15
580		min	-34.606	3	.353	15	-.012	3	0	1	0	1	-.004	4
581	6	max	-.461	10	1.2	4	.022	1	0	3	0	3	-.001	15
582		min	-34.546	3	.282	15	-.012	3	0	1	0	1	-.004	4
583	7	max	-.395	10	.9	4	.022	1	0	3	0	3	-.001	15
584		min	-34.487	3	.212	15	-.012	3	0	1	0	1	-.005	4
585	8	max	-.328	10	.6	4	.022	1	0	3	0	3	-.001	15
586		min	-34.427	3	.141	15	-.012	3	0	1	0	1	-.005	4
587	9	max	-.262	10	.3	4	.022	1	0	3	0	3	-.001	15
588		min	-34.367	3	.071	15	-.012	3	0	1	0	1	-.005	4
589	10	max	-.196	10	0	1	.022	1	0	3	0	3	-.001	15
590		min	-34.308	3	0	1	-.012	3	0	1	0	1	-.006	4
591	11	max	-.129	10	-.071	15	.022	1	0	3	0	3	-.001	15
592		min	-34.248	3	-.3	4	-.012	3	0	1	0	1	-.005	4
593	12	max	-.063	10	-.141	15	.022	1	0	3	0	3	-.001	15
594		min	-34.188	3	-.6	4	-.012	3	0	1	0	1	-.005	4
595	13	max	.003	10	-.212	15	.022	1	0	3	0	2	-.001	15
596		min	-34.129	3	-.9	4	-.012	3	0	1	0	4	-.005	4
597	14	max	.069	10	-.282	15	.022	1	0	3	0	1	-.001	15
598		min	-34.069	3	-1.2	4	-.012	3	0	1	0	3	-.004	4
599	15	max	.136	10	-.353	15	.022	1	0	3	0	1	0	15
600		min	-34.009	3	-1.5	4	-.012	3	0	1	0	3	-.004	4
601	16	max	.202	10	-.423	15	.022	1	0	3	0	1	0	15
602		min	-33.95	3	-1.8	4	-.012	3	0	1	0	3	-.003	4
603	17	max	.268	10	-.494	15	.022	1	0	3	0	1	0	15
604		min	-33.89	3	-2.1	4	-.012	3	0	1	0	3	-.002	4
605	18	max	.335	10	-.564	15	.022	1	0	3	0	1	0	15
606		min	-33.83	3	-2.4	4	-.012	3	0	1	0	3	-.001	4
607	19	max	.401	10	-.635	15	.022	1	0	3	0	1	0	1



Company : Schletter, Inc.
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Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-33.771	3	-2.7	4	-.012	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC	
1	M2	1	max	.003	1	.007	2	.014	1	-3.657e-5	15	NC	3	NC	3	
2			min	-.003	3	-.006	3	0	3	-1.087e-3	1	4477.131	2	2424.803	1	
3			2	max	.003	1	.007	2	.013	1	-3.506e-5	15	NC	3	NC	3
4				min	-.003	3	-.006	3	0	3	-1.042e-3	1	4854.968	2	2624.398	1
5			3	max	.003	1	.006	2	.012	1	-3.356e-5	15	NC	3	NC	3
6				min	-.003	3	-.006	3	0	3	-9.979e-4	1	5298.992	2	2859.441	1
7			4	max	.003	1	.006	2	.011	1	-3.205e-5	15	NC	3	NC	3
8				min	-.002	3	-.006	3	0	3	-9.535e-4	1	5824.276	2	3138.536	1
9			5	max	.002	1	.005	2	.01	1	-3.054e-5	15	NC	3	NC	3
10				min	-.002	3	-.005	3	0	3	-9.091e-4	1	6450.659	2	3473.17	1
11		6	max	.002	1	.005	2	.009	1	-2.904e-5	15	NC	1	NC	3	
12			min	-.002	3	-.005	3	0	3	-8.647e-4	1	7204.674	2	3878.952	1	
13		7	max	.002	1	.004	2	.008	1	-2.753e-5	15	NC	1	NC	2	
14			min	-.002	3	-.005	3	0	3	-8.203e-4	1	8122.47	2	4377.543	1	
15		8	max	.002	1	.004	2	.007	1	-2.602e-5	15	NC	1	NC	2	
16			min	-.002	3	-.005	3	0	3	-7.759e-4	1	9254.38	2	4999.73	1	
17		9	max	.002	1	.003	2	.006	1	-2.451e-5	15	NC	1	NC	2	
18			min	-.002	3	-.004	3	0	3	-7.315e-4	1	NC	1	5790.511	1	
19		10	max	.002	1	.003	2	.005	1	-2.301e-5	15	NC	1	NC	2	
20			min	-.001	3	-.004	3	0	3	-6.872e-4	1	NC	1	6817.866	1	
21		11	max	.001	1	.002	2	.004	1	-2.15e-5	15	NC	1	NC	2	
22			min	-.001	3	-.004	3	0	3	-6.428e-4	1	NC	1	8188.638	1	
23		12	max	.001	1	.002	2	.003	1	-1.999e-5	15	NC	1	NC	1	
24			min	-.001	3	-.003	3	0	3	-5.984e-4	1	NC	1	NC	1	
25		13	max	.001	1	.001	2	.003	1	-1.849e-5	15	NC	1	NC	1	
26			min	0	3	-.003	3	0	3	-5.54e-4	1	NC	1	NC	1	
27		14	max	0	1	.001	2	.002	1	-1.698e-5	15	NC	1	NC	1	
28			min	0	3	-.002	3	0	3	-5.096e-4	1	NC	1	NC	1	
29		15	max	0	1	0	2	.001	1	-1.547e-5	15	NC	1	NC	1	
30			min	0	3	-.002	3	0	3	-4.652e-4	1	NC	1	NC	1	
31		16	max	0	1	0	2	0	1	-1.396e-5	15	NC	1	NC	1	
32			min	0	3	-.001	3	0	3	-4.208e-4	1	NC	1	NC	1	
33		17	max	0	1	0	2	0	1	-1.246e-5	15	NC	1	NC	1	
34			min	0	3	-.001	3	0	3	-3.764e-4	1	NC	1	NC	1	
35		18	max	0	1	0	2	0	1	-1.095e-5	15	NC	1	NC	1	
36			min	0	3	0	3	0	12	-3.32e-4	1	NC	1	NC	1	
37		19	max	0	1	0	1	0	1	-8.33e-6	12	NC	1	NC	1	
38			min	0	1	0	1	0	1	-2.876e-4	1	NC	1	NC	1	
39	M3	1	max	0	1	0	1	0	1	1.322e-4	1	NC	1	NC	1	
40			min	0	1	0	1	0	1	3.94e-6	12	NC	1	NC	1	
41			2	max	0	9	0	2	0	12	1.663e-4	1	NC	1	NC	1
42				min	0	2	0	3	0	1	5.497e-6	15	NC	1	NC	1
43			3	max	0	9	0	2	0	12	2.004e-4	1	NC	1	NC	1
44				min	0	2	-.002	3	0	1	6.653e-6	15	NC	1	NC	1
45			4	max	0	9	0	2	0	12	2.345e-4	1	NC	1	NC	1
46				min	0	2	-.002	3	-.001	1	7.809e-6	15	NC	1	NC	1
47			5	max	0	9	0	2	0	12	2.685e-4	1	NC	1	NC	1
48				min	0	2	-.003	3	-.001	1	8.965e-6	15	NC	1	NC	1
49			6	max	0	9	0	2	0	3	3.026e-4	1	NC	1	NC	1
50				min	0	2	-.004	3	-.001	1	1.012e-5	15	NC	1	NC	1
51		7	max	0	9	0	2	0	3	3.367e-4	1	NC	1	NC	1	



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Designer : HCV
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Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.004	3	0	1	1.128e-5	15	NC	1	NC	1
53	8	max	0	9	.001	2	0	3	3.708e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	1.243e-5	15	NC	1	NC	1
55	9	max	0	9	.002	2	0	3	4.049e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	1	1.359e-5	15	NC	1	NC	1
57	10	max	0	9	.002	2	0	1	4.39e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	15	1.474e-5	15	NC	1	NC	1
59	11	max	0	9	.003	2	.001	1	4.731e-4	1	NC	1	NC	1
60		min	0	2	-.006	3	0	15	1.59e-5	15	NC	1	NC	1
61	12	max	0	9	.003	2	.002	1	5.072e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	1.706e-5	15	NC	1	NC	1
63	13	max	0	9	.004	2	.003	1	5.413e-4	1	NC	1	NC	1
64		min	0	2	-.007	3	0	15	1.821e-5	15	NC	1	NC	1
65	14	max	0	9	.005	2	.003	1	5.754e-4	1	NC	1	NC	1
66		min	0	2	-.007	3	0	15	1.937e-5	15	9814.131	2	NC	1
67	15	max	0	9	.006	2	.004	1	6.095e-4	1	NC	1	NC	1
68		min	0	2	-.007	3	0	15	2.052e-5	15	8272.569	2	NC	1
69	16	max	0	9	.007	2	.005	1	6.435e-4	1	NC	3	NC	2
70		min	0	2	-.007	3	0	15	2.168e-5	15	7073.684	2	9494.997	1
71	17	max	0	9	.008	2	.006	1	6.776e-4	1	NC	3	NC	2
72		min	0	2	-.008	3	0	15	2.284e-5	15	6132.037	2	8197.512	1
73	18	max	.001	9	.009	2	.006	1	7.117e-4	1	NC	3	NC	2
74		min	0	2	-.008	3	0	15	2.399e-5	15	5386.065	2	7244.658	1
75	19	max	.001	9	.01	2	.007	1	7.458e-4	1	NC	3	NC	2
76		min	0	2	-.008	3	0	15	2.515e-5	15	4791.126	2	6530.444	1
77	M4	1	max	.002	1	.009	2	15	-3.023e-5	15	NC	1	NC	2
78		min	0	3	-.007	3	-.005	1	-9.086e-4	1	NC	1	3664.513	1
79	2	max	.002	1	.008	2	0	15	-3.023e-5	15	NC	1	NC	2
80		min	0	3	-.006	3	-.005	1	-9.086e-4	1	NC	1	3997.191	1
81	3	max	.002	1	.008	2	0	15	-3.023e-5	15	NC	1	NC	2
82		min	0	3	-.006	3	-.004	1	-9.086e-4	1	NC	1	4393.153	1
83	4	max	.002	1	.007	2	0	15	-3.023e-5	15	NC	1	NC	2
84		min	0	3	-.006	3	-.004	1	-9.086e-4	1	NC	1	4869.087	1
85	5	max	.002	1	.007	2	0	15	-3.023e-5	15	NC	1	NC	2
86		min	0	3	-.005	3	-.004	1	-9.086e-4	1	NC	1	5447.727	1
87	6	max	.002	1	.006	2	0	15	-3.023e-5	15	NC	1	NC	2
88		min	0	3	-.005	3	-.003	1	-9.086e-4	1	NC	1	6160.693	1
89	7	max	.002	1	.006	2	0	15	-3.023e-5	15	NC	1	NC	2
90		min	0	3	-.004	3	-.003	1	-9.086e-4	1	NC	1	7053.027	1
91	8	max	.002	1	.005	2	0	15	-3.023e-5	15	NC	1	NC	2
92		min	0	3	-.004	3	-.002	1	-9.086e-4	1	NC	1	8190.687	1
93	9	max	.001	1	.005	2	0	15	-3.023e-5	15	NC	1	NC	2
94		min	0	3	-.004	3	-.002	1	-9.086e-4	1	NC	1	9673.452	1
95	10	max	.001	1	.004	2	0	15	-3.023e-5	15	NC	1	NC	1
96		min	0	3	-.003	3	-.002	1	-9.086e-4	1	NC	1	NC	1
97	11	max	.001	1	.004	2	0	15	-3.023e-5	15	NC	1	NC	1
98		min	0	3	-.003	3	-.001	1	-9.086e-4	1	NC	1	NC	1
99	12	max	0	1	.003	2	0	15	-3.023e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	-.001	1	-9.086e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-3.023e-5	15	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-9.086e-4	1	NC	1	NC	1
103	14	max	0	1	.002	2	0	15	-3.023e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-9.086e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-3.023e-5	15	NC	1	NC	1
106		min	0	3	-.001	3	0	1	-9.086e-4	1	NC	1	NC	1
107	16	max	0	1	.001	2	0	15	-3.023e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-9.086e-4	1	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-3.023e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-9.086e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.023e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-9.086e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.023e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.086e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.024	2	.004	1	1.949e-4	3	NC	3	NC	2
116			min	-.01	3	-.018	3	-.002	3	2.485e-6	10	1395.851	2	7815.263	1
117		2	max	.009	1	.022	2	.004	1	1.896e-4	3	NC	3	NC	2
118			min	-.009	3	-.017	3	-.002	3	1.781e-6	10	1490.338	2	8474.696	1
119		3	max	.009	1	.021	2	.004	1	1.843e-4	3	NC	3	NC	2
120			min	-.009	3	-.016	3	-.002	3	1.078e-6	10	1598.199	2	9256.4	1
121		4	max	.008	1	.019	2	.003	1	1.79e-4	3	NC	3	NC	1
122			min	-.008	3	-.015	3	-.002	3	3.74e-7	10	1722.109	2	NC	1
123		5	max	.008	1	.018	2	.003	1	1.738e-4	3	NC	3	NC	1
124			min	-.007	3	-.014	3	-.002	3	-3.297e-7	10	1865.511	2	NC	1
125		6	max	.007	1	.016	2	.003	1	1.685e-4	3	NC	3	NC	1
126			min	-.007	3	-.013	3	-.002	3	-1.033e-6	10	2032.914	2	NC	1
127		7	max	.007	1	.015	2	.002	1	1.632e-4	3	NC	3	NC	1
128			min	-.006	3	-.013	3	-.001	3	-3.452e-6	2	2230.331	2	NC	1
129		8	max	.006	1	.013	2	.002	1	1.579e-4	3	NC	3	NC	1
130			min	-.006	3	-.012	3	-.001	3	-7.098e-6	2	2465.968	2	NC	1
131		9	max	.006	1	.012	2	.002	1	1.526e-4	3	NC	3	NC	1
132			min	-.005	3	-.011	3	-.001	3	-1.074e-5	2	2751.315	2	NC	1
133		10	max	.005	1	.011	2	.001	1	1.473e-4	3	NC	3	NC	1
134			min	-.005	3	-.01	3	0	3	-1.439e-5	2	3102.974	2	NC	1
135		11	max	.004	1	.009	2	.001	1	1.42e-4	3	NC	3	NC	1
136			min	-.004	3	-.009	3	0	3	-1.804e-5	2	3545.845	2	NC	1
137		12	max	.004	1	.008	2	0	1	1.367e-4	3	NC	3	NC	1
138			min	-.004	3	-.008	3	0	3	-2.168e-5	2	4119.06	2	NC	1
139		13	max	.003	1	.007	2	0	1	1.314e-4	3	NC	3	NC	1
140			min	-.003	3	-.007	3	0	3	-2.533e-5	2	4887.837	2	NC	1
141		14	max	.003	1	.006	2	0	1	1.261e-4	3	NC	3	NC	1
142			min	-.003	3	-.006	3	0	3	-2.897e-5	2	5969.558	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.208e-4	3	NC	3	NC	1
144			min	-.002	3	-.004	3	0	3	-3.262e-5	2	7598.98	2	NC	1
145		16	max	.002	1	.003	2	0	1	1.155e-4	3	NC	1	NC	1
146			min	-.002	3	-.003	3	0	3	-3.627e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	1.102e-4	3	NC	1	NC	1
148			min	-.001	3	-.002	3	0	3	-3.991e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.049e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-4.356e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	9.964e-5	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.721e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.146e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-4.559e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.916e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	2	-3.436e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.823e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-2.312e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.73e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.188e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	0	3	1.637e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	1	-6.481e-7	3	8793.896	2	NC	1
163		6	max	0	3	.007	2	0	3	1.543e-5	1	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	4.473e-7	15	7046.547	2	NC	1
165		7	max	0	3	.008	2	.001	3	2.182e-5	3	NC	3	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
166			min	-.001	2	-.009	3	0	1	4.778e-7	15	5851.203	2	NC	1
167		8	max	.001	3	.009	2	.001	3	3.306e-5	3	NC	3	NC	1
168			min	-.001	2	-.011	3	0	1	-2.693e-6	2	4974.658	2	NC	1
169		9	max	.001	3	.011	2	.001	3	4.429e-5	3	NC	3	NC	1
170			min	-.002	2	-.012	3	0	1	-6.144e-6	2	4300.729	2	NC	1
171		10	max	.001	3	.012	2	.001	3	5.553e-5	3	NC	3	NC	1
172			min	-.002	2	-.013	3	-.001	1	-9.594e-6	2	3765.011	2	NC	1
173		11	max	.002	3	.014	2	.001	3	6.677e-5	3	NC	3	NC	1
174			min	-.002	2	-.014	3	-.001	1	-1.304e-5	2	3328.818	2	NC	1
175		12	max	.002	3	.016	2	.001	3	7.8e-5	3	NC	3	NC	1
176			min	-.002	2	-.015	3	-.001	1	-1.649e-5	2	2967.38	2	NC	1
177		13	max	.002	3	.017	2	.002	3	8.924e-5	3	NC	3	NC	1
178			min	-.002	2	-.016	3	-.002	1	-1.995e-5	2	2663.972	2	NC	1
179		14	max	.002	3	.019	2	.002	3	1.005e-4	3	NC	3	NC	1
180			min	-.003	2	-.017	3	-.002	1	-2.34e-5	2	2406.794	2	NC	1
181		15	max	.002	3	.021	2	.001	3	1.117e-4	3	NC	3	NC	1
182			min	-.003	2	-.018	3	-.002	1	-2.685e-5	2	2187.204	2	NC	1
183		16	max	.002	3	.023	2	.001	3	1.229e-4	3	NC	3	NC	1
184			min	-.003	2	-.019	3	-.002	1	-3.03e-5	2	1998.68	2	NC	1
185		17	max	.003	3	.025	2	.001	3	1.342e-4	3	NC	3	NC	1
186			min	-.003	2	-.02	3	-.002	1	-3.375e-5	2	1836.174	2	NC	1
187		18	max	.003	3	.027	2	.001	3	1.454e-4	3	NC	3	NC	1
188			min	-.003	2	-.02	3	-.002	1	-3.72e-5	2	1695.708	2	NC	1
189		19	max	.003	3	.029	2	.001	3	1.567e-4	3	NC	3	NC	1
190			min	-.004	2	-.021	3	-.002	1	-4.065e-5	2	1574.099	2	NC	1
191	M8	1	max	.007	1	.027	2	.002	1	-1.596e-6	10	NC	1	NC	2
192			min	-.001	3	-.019	3	0	3	-1.232e-4	3	NC	1	7989.377	1
193		2	max	.007	1	.026	2	.002	1	-1.596e-6	10	NC	1	NC	2
194			min	-.001	3	-.018	3	0	3	-1.232e-4	3	NC	1	8710.603	1
195		3	max	.006	1	.024	2	.002	1	-1.596e-6	10	NC	1	NC	2
196			min	-.001	3	-.017	3	0	3	-1.232e-4	3	NC	1	9569.223	1
197		4	max	.006	1	.023	2	.002	1	-1.596e-6	10	NC	1	NC	1
198			min	-.001	3	-.016	3	0	3	-1.232e-4	3	NC	1	NC	1
199		5	max	.006	1	.021	2	.002	1	-1.596e-6	10	NC	1	NC	1
200			min	0	3	-.015	3	0	3	-1.232e-4	3	NC	1	NC	1
201		6	max	.005	1	.02	2	.001	1	-1.596e-6	10	NC	1	NC	1
202			min	0	3	-.014	3	0	3	-1.232e-4	3	NC	1	NC	1
203		7	max	.005	1	.018	2	.001	1	-1.596e-6	10	NC	1	NC	1
204			min	0	3	-.013	3	0	3	-1.232e-4	3	NC	1	NC	1
205		8	max	.004	1	.017	2	.001	1	-1.596e-6	10	NC	1	NC	1
206			min	0	3	-.012	3	0	3	-1.232e-4	3	NC	1	NC	1
207		9	max	.004	1	.015	2	0	1	-1.596e-6	10	NC	1	NC	1
208			min	0	3	-.01	3	0	3	-1.232e-4	3	NC	1	NC	1
209		10	max	.004	1	.014	2	0	1	-1.596e-6	10	NC	1	NC	1
210			min	0	3	-.009	3	0	3	-1.232e-4	3	NC	1	NC	1
211		11	max	.003	1	.012	2	0	1	-1.596e-6	10	NC	1	NC	1
212			min	0	3	-.008	3	0	3	-1.232e-4	3	NC	1	NC	1
213		12	max	.003	1	.011	2	0	1	-1.596e-6	10	NC	1	NC	1
214			min	0	3	-.007	3	0	3	-1.232e-4	3	NC	1	NC	1
215		13	max	.002	1	.009	2	0	1	-1.596e-6	10	NC	1	NC	1
216			min	0	3	-.006	3	0	3	-1.232e-4	3	NC	1	NC	1
217		14	max	.002	1	.008	2	0	1	-1.596e-6	10	NC	1	NC	1
218			min	0	3	-.005	3	0	3	-1.232e-4	3	NC	1	NC	1
219		15	max	.002	1	.006	2	0	1	-1.596e-6	10	NC	1	NC	1
220			min	0	3	-.004	3	0	3	-1.232e-4	3	NC	1	NC	1
221		16	max	.001	1	.005	2	0	1	-1.596e-6	10	NC	1	NC	1
222			min	0	3	-.003	3	0	3	-1.232e-4	3	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-1.596e-6	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.232e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.596e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.232e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.596e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.232e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.007	2	0	3	9.479e-4	1	NC	3	NC	1
230			min	-.003	3	-.006	3	-.002	1	-2.003e-4	3	4482.657	2	NC	1
231		2	max	.003	1	.007	2	0	3	8.99e-4	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.002	1	-1.946e-4	3	4861.082	2	NC	1
233		3	max	.003	1	.006	2	0	3	8.5e-4	1	NC	3	NC	1
234			min	-.003	3	-.006	3	-.002	1	-1.89e-4	3	5305.818	2	NC	1
235		4	max	.003	1	.006	2	0	3	8.011e-4	1	NC	3	NC	1
236			min	-.002	3	-.006	3	-.002	1	-1.834e-4	3	5831.97	2	NC	1
237		5	max	.002	1	.005	2	0	3	7.522e-4	1	NC	3	NC	1
238			min	-.002	3	-.005	3	-.001	1	-1.778e-4	3	6459.423	2	NC	1
239		6	max	.002	1	.005	2	0	3	7.033e-4	1	NC	1	NC	1
240			min	-.002	3	-.005	3	-.001	1	-1.721e-4	3	7214.769	2	NC	1
241		7	max	.002	1	.004	2	0	3	6.543e-4	1	NC	1	NC	1
242			min	-.002	3	-.005	3	-.001	1	-1.665e-4	3	8134.244	2	NC	1
243		8	max	.002	1	.004	2	0	3	6.054e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	-.001	1	-1.609e-4	3	9268.3	2	NC	1
245		9	max	.002	1	.003	2	0	3	5.565e-4	1	NC	1	NC	1
246			min	-.002	3	-.004	3	0	1	-1.552e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	5.076e-4	1	NC	1	NC	1
248			min	-.001	3	-.004	3	0	1	-1.496e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.586e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.44e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.097e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.384e-4	3	NC	1	NC	1
253		13	max	.001	1	.001	2	0	3	3.608e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-1.327e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.119e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.271e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.629e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.215e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.14e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.159e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.651e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-1.102e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.162e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.046e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.723e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-9.898e-5	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	4.554e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.226e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	1	3.251e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-9.296e-5	1	NC	1	NC	1
271		3	max	0	9	0	2	0	10	1.948e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.537e-4	1	NC	1	NC	1
273		4	max	0	9	0	2	0	10	6.456e-6	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-2.144e-4	1	NC	1	NC	1
275		5	max	0	9	0	2	0	10	-4.691e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	0	1	-2.751e-4	1	NC	1	NC	1
277		6	max	0	9	0	2	0	15	-1.121e-5	15	NC	1	NC	1
278			min	0	2	-.004	3	-.001	1	-3.358e-4	1	NC	1	NC	1
279		7	max	0	9	0	2	0	15	-1.329e-5	15	NC	1	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	9.746e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.372e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	9.746e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.372e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	9.746e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.372e-5	15	NC	1	NC	1
343	M1	1	max	.006	3	.023	3	.001	3	1.559e-2	1	NC	1	NC	1
344			min	-.007	2	-.028	1	-.005	1	-1.408e-2	3	NC	1	NC	1
345		2	max	.006	3	.013	3	0	3	7.409e-3	1	NC	4	NC	2
346			min	-.007	2	-.015	1	-.01	1	-6.967e-3	3	3563.255	1	8394.162	1
347		3	max	.006	3	.003	3	0	3	1.474e-5	3	NC	4	NC	2
348			min	-.007	2	-.003	1	-.014	1	-6.211e-4	1	1843.013	1	5089.783	1
349		4	max	.006	3	.007	1	0	3	1.796e-5	3	NC	5	NC	2
350			min	-.007	2	-.005	3	-.016	1	-5.188e-4	1	1303.761	1	4211.084	1
351		5	max	.006	3	.016	1	0	3	2.118e-5	3	NC	5	NC	2
352			min	-.007	2	-.011	3	-.016	1	-4.165e-4	1	1044.597	1	4042.655	1
353		6	max	.006	3	.023	1	0	3	2.44e-5	3	NC	5	NC	2
354			min	-.007	2	-.017	3	-.015	1	-3.142e-4	1	898.151	1	4324.029	1
355		7	max	.006	3	.029	1	0	3	2.762e-5	3	NC	5	NC	2
356			min	-.007	2	-.02	3	-.013	1	-2.119e-4	1	809.529	1	5145.139	1
357		8	max	.006	3	.033	1	0	3	3.085e-5	3	NC	5	NC	2
358			min	-.007	2	-.023	3	-.011	1	-1.096e-4	1	755.906	1	7053.298	1
359		9	max	.006	3	.035	1	0	3	3.407e-5	3	NC	5	NC	1
360			min	-.007	2	-.025	3	-.008	1	-9.63e-6	2	726.629	1	NC	1
361		10	max	.006	3	.036	1	0	3	9.506e-5	1	NC	5	NC	1
362			min	-.007	2	-.025	3	-.005	1	3.477e-6	15	716.785	1	NC	1
363		11	max	.006	3	.035	1	0	3	1.974e-4	1	NC	5	NC	1
364			min	-.007	2	-.024	3	-.001	1	6.894e-6	15	724.888	1	NC	1
365		12	max	.006	3	.032	1	.002	1	2.997e-4	1	NC	5	NC	2
366			min	-.007	2	-.022	3	0	15	1.031e-5	15	752.256	1	8162.536	1
367		13	max	.006	3	.028	1	.004	1	4.02e-4	1	NC	5	NC	2
368			min	-.007	2	-.019	3	0	15	1.373e-5	15	803.573	1	5669.454	1
369		14	max	.006	3	.023	1	.006	1	5.043e-4	1	NC	5	NC	2
370			min	-.007	2	-.015	3	0	15	1.715e-5	15	889.109	1	4648.986	1
371		15	max	.006	3	.015	1	.007	1	6.066e-4	1	NC	5	NC	2
372			min	-.007	2	-.01	3	0	15	2.057e-5	15	1030.887	1	4282.229	1
373		16	max	.006	3	.006	1	.007	1	6.807e-4	1	NC	5	NC	2
374			min	-.007	2	-.004	3	0	15	2.306e-5	15	1281.446	1	4413.503	1
375		17	max	.006	3	.002	3	.005	1	8.345e-5	1	NC	4	NC	2
376			min	-.007	2	-.004	2	0	15	3.499e-6	15	1798.905	1	5295.216	1
377		18	max	.006	3	.01	3	.002	1	8.786e-3	1	NC	4	NC	2
378			min	-.007	2	-.017	1	0	15	-3.222e-3	3	3467.003	1	8688.589	1
379		19	max	.006	3	.018	3	0	3	1.766e-2	1	NC	1	NC	1
380			min	-.007	2	-.031	1	-.003	1	-6.534e-3	3	NC	1	NC	1
381	M5	1	max	.018	3	.068	3	.001	3	6.734e-7	3	NC	1	NC	1
382			min	-.023	2	-.085	1	-.005	1	4.254e-8	15	NC	1	NC	1
383		2	max	.018	3	.038	3	.002	3	5.027e-5	3	NC	5	NC	1
384			min	-.023	2	-.047	1	-.005	1	-8.423e-5	1	1207.273	1	NC	1
385		3	max	.018	3	.01	3	.002	3	9.892e-5	3	NC	5	NC	1
386			min	-.023	2	-.01	1	-.004	1	-1.672e-4	1	621.4	1	NC	1
387		4	max	.018	3	.02	1	.003	3	9.741e-5	3	NC	5	NC	1
388			min	-.023	2	-.013	3	-.004	1	-1.57e-4	1	438.435	1	NC	1
389		5	max	.018	3	.047	1	.003	3	9.59e-5	3	NC	15	NC	1
390			min	-.023	2	-.032	3	-.003	1	-1.468e-4	1	350.459	1	NC	1
391		6	max	.017	3	.068	1	.003	3	9.439e-5	3	NC	15	NC	1
392			min	-.023	2	-.047	3	-.003	1	-1.365e-4	1	300.665	1	NC	1
393		7	max	.017	3	.085	1	.003	3	9.289e-5	3	NC	15	NC	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
394			min	-.023	2	-.058	3	-.003	1	-1.263e-4	1	270.433	1	NC	1
395		8	max	.017	3	.098	1	.003	3	9.138e-5	3	NC	15	NC	1
396			min	-.023	2	-.066	3	-.002	1	-1.16e-4	1	252.019	1	NC	1
397		9	max	.017	3	.105	1	.003	3	8.987e-5	3	NC	15	NC	1
398			min	-.023	2	-.07	3	-.002	1	-1.058e-4	1	241.803	1	NC	1
399		10	max	.017	3	.107	1	.003	3	8.837e-5	3	NC	15	NC	1
400			min	-.024	2	-.07	3	-.002	1	-9.555e-5	1	238.106	1	NC	1
401		11	max	.017	3	.105	1	.003	3	8.686e-5	3	NC	15	NC	1
402			min	-.024	2	-.068	3	-.002	1	-8.531e-5	1	240.405	1	NC	1
403		12	max	.017	3	.098	1	.003	3	8.535e-5	3	NC	15	NC	1
404			min	-.024	2	-.062	3	-.002	1	-7.506e-5	1	249.116	1	NC	1
405		13	max	.017	3	.085	1	.002	3	8.385e-5	3	NC	15	NC	1
406			min	-.024	2	-.054	3	-.002	1	-6.482e-5	1	265.78	1	NC	1
407		14	max	.017	3	.068	1	.002	3	8.234e-5	3	NC	15	NC	1
408			min	-.024	2	-.042	3	-.002	1	-5.458e-5	1	293.803	1	NC	1
409		15	max	.017	3	.046	1	.002	3	8.083e-5	3	NC	5	NC	1
410			min	-.024	2	-.028	3	-.002	1	-4.434e-5	1	340.529	1	NC	1
411		16	max	.017	3	.018	1	.001	3	7.708e-5	3	NC	5	NC	1
412			min	-.024	2	-.012	3	-.002	1	-4.135e-5	1	423.609	1	NC	1
413		17	max	.017	3	.007	3	0	3	1.988e-5	3	NC	5	NC	1
414			min	-.024	2	-.014	1	-.002	1	-2.109e-4	1	597.254	1	NC	1
415		18	max	.017	3	.028	3	0	3	9.435e-6	3	NC	5	NC	1
416			min	-.024	2	-.053	1	-.003	1	-1.081e-4	1	1157.529	1	NC	1
417		19	max	.017	3	.05	3	0	3	0	15	NC	1	NC	1
418			min	-.024	2	-.094	1	-.003	1	-1.294e-7	3	NC	1	NC	1
419	M9	1	max	.006	3	.023	3	0	3	1.408e-2	3	NC	1	NC	1
420			min	-.007	2	-.028	1	-.006	1	-1.559e-2	1	NC	1	NC	1
421		2	max	.006	3	.013	3	0	3	6.979e-3	3	NC	4	NC	2
422			min	-.007	2	-.015	1	-.001	1	-7.666e-3	1	3564.13	1	9718.796	1
423		3	max	.006	3	.003	3	.002	1	1.11e-4	1	NC	4	NC	2
424			min	-.007	2	-.003	1	0	3	3.907e-6	15	1843.477	1	6033.638	1
425		4	max	.006	3	.007	1	.004	1	2.5e-5	1	NC	5	NC	2
426			min	-.007	2	-.005	3	0	3	-1.632e-6	3	1304.086	1	5110.954	1
427		5	max	.006	3	.016	1	.004	1	2.993e-6	10	NC	5	NC	2
428			min	-.007	2	-.011	3	-.001	3	-6.099e-5	1	1044.843	1	5062.688	1
429		6	max	.006	3	.023	1	.003	1	-4.051e-6	10	NC	5	NC	2
430			min	-.007	2	-.017	3	-.001	3	-1.47e-4	1	898.347	1	5676.477	1
431		7	max	.006	3	.029	1	.002	1	-7.695e-6	15	NC	5	NC	2
432			min	-.007	2	-.021	3	-.002	3	-2.33e-4	1	809.689	1	7327.307	1
433		8	max	.006	3	.033	1	0	2	-1.06e-5	15	NC	5	NC	1
434			min	-.007	2	-.023	3	-.002	3	-3.189e-4	1	756.04	1	NC	1
435		9	max	.006	3	.035	1	0	10	-1.35e-5	15	NC	5	NC	1
436			min	-.007	2	-.025	3	-.003	1	-4.049e-4	1	726.742	1	NC	1
437		10	max	.006	3	.036	1	0	15	-1.64e-5	15	NC	5	NC	1
438			min	-.007	2	-.025	3	-.006	1	-4.909e-4	1	716.882	1	NC	1
439		11	max	.006	3	.035	1	0	15	-1.93e-5	15	NC	5	NC	2
440			min	-.007	2	-.024	3	-.009	1	-5.769e-4	1	724.97	1	9215.553	1
441		12	max	.006	3	.032	1	0	15	-2.22e-5	15	NC	5	NC	2
442			min	-.007	2	-.022	3	-.011	1	-6.629e-4	1	752.324	1	5986.455	1
443		13	max	.006	3	.028	1	0	15	-2.51e-5	15	NC	5	NC	2
444			min	-.007	2	-.019	3	-.013	1	-7.489e-4	1	803.63	1	4655.924	1
445		14	max	.006	3	.023	1	0	15	-2.8e-5	15	NC	5	NC	2
446			min	-.007	2	-.015	3	-.015	1	-8.349e-4	1	889.154	1	4050.769	1
447		15	max	.006	3	.015	1	0	15	-3.09e-5	15	NC	5	NC	2
448			min	-.007	2	-.01	3	-.015	1	-9.208e-4	1	1030.92	1	3869.234	1
449		16	max	.006	3	.006	1	0	15	-3.304e-5	15	NC	5	NC	2
450			min	-.007	2	-.004	3	-.014	1	-9.85e-4	1	1281.47	1	4086.041	1



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Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.006	3	.002	3	0	15	-1.076e-5	12	NC	4	NC	2
452			min	-.007	2	-.004	2	-.012	1	-5.292e-4	1	1798.938	1	4987.935	1
453		18	max	.006	3	.01	3	0	15	3.229e-3	3	NC	4	NC	2
454			min	-.007	2	-.017	1	-.008	1	-9.013e-3	1	3467.06	1	8289.714	1
455		19	max	.006	3	.018	3	0	3	6.533e-3	3	NC	1	NC	1
456			min	-.007	2	-.031	1	-.002	1	-1.766e-2	1	NC	1	NC	1
457	M13	1	max	.006	1	.023	3	.006	3	3.919e-3	3	NC	1	NC	1
458			min	0	3	-.028	1	-.007	2	-4.962e-3	1	NC	1	NC	1
459		2	max	.006	1	.163	3	.039	1	4.731e-3	3	NC	5	NC	2
460			min	-.001	3	-.184	1	0	10	-6.018e-3	1	1195.81	1	4275.092	1
461		3	max	.006	1	.278	3	.098	1	5.542e-3	3	NC	5	NC	3
462			min	-.001	3	-.311	1	.003	15	-7.074e-3	1	657.117	1	1802.239	1
463		4	max	.006	1	.351	3	.148	1	6.353e-3	3	NC	5	NC	3
464			min	-.001	3	-.392	1	.005	15	-8.129e-3	1	511.517	1	1213.001	1
465		5	max	.006	1	.372	3	.173	1	7.164e-3	3	NC	5	NC	3
466			min	-.001	3	-.417	1	.006	15	-9.185e-3	1	479.014	1	1048.023	1
467		6	max	.006	1	.344	3	.164	1	7.976e-3	3	NC	5	NC	3
468			min	-.001	3	-.386	1	.006	15	-1.024e-2	1	519.449	1	1103.798	1
469		7	max	.006	1	.276	3	.124	1	8.787e-3	3	NC	5	NC	3
470			min	-.001	3	-.312	1	.002	10	-1.13e-2	1	655.16	1	1445.847	1
471		8	max	.006	1	.187	3	.064	1	9.598e-3	3	NC	5	NC	3
472			min	-.001	3	-.215	1	-.004	10	-1.235e-2	1	996.306	1	2679.597	1
473		9	max	.005	1	.105	3	.016	3	1.041e-2	3	NC	4	NC	1
474			min	-.001	3	-.126	1	-.012	2	-1.341e-2	1	1911.61	1	NC	1
475		10	max	.005	1	.068	3	.018	3	1.122e-2	3	NC	4	NC	1
476			min	-.001	3	-.085	1	-.023	2	-1.446e-2	1	3289.295	1	NC	1
477		11	max	.005	1	.105	3	.02	3	1.041e-2	3	NC	4	NC	1
478			min	-.001	3	-.126	1	-.012	2	-1.341e-2	1	1911.611	1	NC	1
479		12	max	.005	1	.187	3	.07	1	9.599e-3	3	NC	5	NC	3
480			min	-.001	3	-.215	1	-.004	10	-1.235e-2	1	996.307	1	2472.671	1
481		13	max	.005	1	.276	3	.131	1	8.788e-3	3	NC	5	NC	5
482			min	-.001	3	-.312	1	.002	10	-1.13e-2	1	655.16	1	1372.157	1
483		14	max	.005	1	.344	3	.171	1	7.977e-3	3	NC	5	NC	5
484			min	-.001	3	-.386	1	.006	15	-1.024e-2	1	519.449	1	1059.448	1
485		15	max	.005	1	.372	3	.179	1	7.166e-3	3	NC	5	NC	3
486			min	-.001	3	-.417	1	.006	15	-9.184e-3	1	479.015	1	1011.37	1
487		16	max	.005	1	.351	3	.154	1	6.355e-3	3	NC	5	NC	3
488			min	-.001	3	-.392	1	.005	15	-8.129e-3	1	511.517	1	1173.002	1
489		17	max	.005	1	.278	3	.102	1	5.545e-3	3	NC	5	NC	3
490			min	-.001	3	-.311	1	.004	15	-7.073e-3	1	657.118	1	1740.981	1
491		18	max	.005	1	.163	3	.04	1	4.734e-3	3	NC	5	NC	2
492			min	-.001	3	-.184	1	0	10	-6.017e-3	1	1195.811	1	4102.878	1
493		19	max	.005	1	.023	3	.006	3	3.923e-3	3	NC	1	NC	1
494			min	-.001	3	-.028	1	-.007	2	-4.961e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.018	3	.006	3	5.197e-3	1	NC	1	NC	1
496			min	0	3	-.031	1	-.007	2	-2.984e-3	3	NC	1	NC	1
497		2	max	.002	1	.083	3	.041	1	6.337e-3	1	NC	5	NC	2
498			min	0	3	-.207	1	0	10	-3.572e-3	3	1055.471	1	4018.299	1
499		3	max	.002	1	.137	3	.103	1	7.477e-3	1	NC	5	NC	3
500			min	0	3	-.351	1	.004	15	-4.159e-3	3	580.047	1	1723.072	1
501		4	max	.002	1	.171	3	.154	1	8.617e-3	1	NC	5	NC	3
502			min	0	3	-.443	1	.005	15	-4.746e-3	3	451.592	1	1167.826	1
503		5	max	.002	1	.182	3	.179	1	9.757e-3	1	NC	5	NC	3
504			min	0	3	-.47	1	.006	15	-5.334e-3	3	423.005	1	1011.559	1
505		6	max	.002	1	.171	3	.169	1	1.09e-2	1	NC	5	NC	5
506			min	0	3	-.436	1	.006	15	-5.921e-3	3	458.915	1	1064.706	1
507		7	max	.002	1	.142	3	.129	1	1.204e-2	1	NC	5	NC	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-352	1	.002	10	-6.508e-3	3	579.294	1	1388.143	1
509	8	max	.003	1	.103	3	.068	1	1.318e-2	1	NC	5	NC	3
510		min	0	3	-.241	1	-.004	10	-7.096e-3	3	882.544	1	2536.123	1
511	9	max	.003	1	.067	3	.019	3	1.432e-2	1	NC	4	NC	1
512		min	0	3	-.14	1	-.013	2	-7.683e-3	3	1701.296	1	NC	1
513	10	max	.003	1	.05	3	.017	3	1.546e-2	1	NC	4	NC	1
514		min	0	3	-.094	1	-.024	2	-8.271e-3	3	2948.025	1	NC	1
515	11	max	.003	1	.067	3	.017	3	1.432e-2	1	NC	4	NC	1
516		min	0	3	-.14	1	-.012	2	-7.683e-3	3	1701.296	1	NC	1
517	12	max	.003	1	.103	3	.066	1	1.318e-2	1	NC	5	NC	3
518		min	0	3	-.241	1	-.004	10	-7.095e-3	3	882.544	1	2607.979	1
519	13	max	.003	1	.142	3	.126	1	1.204e-2	1	NC	5	NC	3
520		min	0	3	-352	1	.002	10	-6.507e-3	3	579.294	1	1418.389	1
521	14	max	.003	1	.171	3	.166	1	1.09e-2	1	NC	5	NC	3
522		min	0	3	-.436	1	.006	15	-5.919e-3	3	458.915	1	1086.237	1
523	15	max	.003	1	.182	3	.175	1	9.758e-3	1	NC	5	NC	3
524		min	0	3	-.47	1	.006	15	-5.332e-3	3	423.006	1	1032.622	1
525	16	max	.003	1	.171	3	.15	1	8.618e-3	1	NC	5	NC	3
526		min	0	3	-.443	1	.005	15	-4.744e-3	3	451.592	1	1195.1	1
527	17	max	.003	1	.137	3	.1	1	7.479e-3	1	NC	5	NC	3
528		min	0	3	-.351	1	.003	15	-4.156e-3	3	580.048	1	1772.811	1
529	18	max	.003	1	.083	3	.039	1	6.339e-3	1	NC	5	NC	2
530		min	0	3	-.207	1	0	10	-3.568e-3	3	1055.472	1	4185.19	1
531	19	max	.003	1	.018	3	.006	3	5.2e-3	1	NC	1	NC	1
532		min	0	3	-.031	1	-.007	2	-2.98e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.17e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-8.484e-5	2	NC	1	NC	1
535	2	max	0	3	-.005	15	.001	1	8.028e-4	3	NC	5	NC	1
536		min	0	10	-.021	4	0	3	-7.403e-4	1	4688.414	4	NC	1
537	3	max	0	3	-.01	15	.004	1	1.289e-3	3	NC	15	NC	1
538		min	0	10	-.042	4	-.003	3	-1.41e-3	1	2385.774	4	NC	1
539	4	max	0	3	-.014	15	.008	1	1.774e-3	3	6963.098	15	NC	3
540		min	0	10	-.061	4	-.006	3	-2.08e-3	1	1636.78	4	8397.336	1
541	5	max	0	3	-.018	15	.013	1	2.26e-3	3	5433.376	15	NC	4
542		min	0	10	-.078	4	-.01	3	-2.75e-3	1	1277.196	4	5505.384	1
543	6	max	0	3	-.022	15	.018	1	2.746e-3	3	4572.76	15	NC	4
544		min	0	10	-.092	4	-.014	3	-3.419e-3	1	1074.896	4	4005.561	1
545	7	max	0	3	-.024	15	.024	1	3.232e-3	3	4055.213	15	NC	4
546		min	0	10	-.104	4	-.019	3	-4.089e-3	1	953.238	4	3129.503	1
547	8	max	0	3	-.026	15	.03	1	3.718e-3	3	3744.607	15	NC	4
548		min	0	10	-.113	4	-.023	3	-4.759e-3	1	880.226	4	2579.201	1
549	9	max	0	3	-.028	15	.035	1	4.203e-3	3	3577.421	15	NC	4
550		min	0	10	-.118	4	-.027	3	-5.429e-3	1	840.926	4	2219.255	1
551	10	max	0	3	-.028	15	.039	1	4.689e-3	3	3524.531	15	NC	4
552		min	0	10	-.12	4	-.031	3	-6.098e-3	1	828.494	4	1981.727	1
553	11	max	0	3	-.028	15	.041	1	5.175e-3	3	3577.421	15	NC	5
554		min	0	10	-.118	4	-.033	3	-6.768e-3	1	840.926	4	1830.967	1
555	12	max	0	3	-.026	15	.043	1	5.661e-3	3	3744.607	15	NC	5
556		min	0	10	-.113	4	-.034	3	-7.438e-3	1	880.226	4	1749.274	1
557	13	max	0	3	-.024	15	.042	1	6.146e-3	3	4055.213	15	NC	5
558		min	0	10	-.105	4	-.033	3	-8.108e-3	1	953.238	4	1731.465	1
559	14	max	0	3	-.022	15	.039	1	6.632e-3	3	4572.76	15	NC	5
560		min	0	10	-.093	4	-.031	3	-8.777e-3	1	1074.896	4	1785.027	1
561	15	max	0	3	-.018	15	.034	1	7.118e-3	3	5433.376	15	NC	4
562		min	0	10	-.078	4	-.026	3	-9.447e-3	1	1277.196	4	1937.637	1
563	16	max	0	3	-.014	15	.025	1	7.604e-3	3	6963.098	15	NC	4
564		min	0	10	-.062	4	-.019	3	-1.012e-2	1	1636.78	4	2264.53	1

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...]	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565		17	max	0	3	-.01	15	.013	1	8.09e-3	3	NC	15	NC	4
566			min	0	10	-.043	4	-.009	3	-1.079e-2	1	2385.774	4	3001.803	1
567		18	max	0	3	-.005	15	.004	3	8.575e-3	3	NC	5	NC	4
568			min	0	10	-.022	4	-.008	2	-1.146e-2	1	4688.414	4	5343.879	1
569		19	max	0	3	.004	3	.02	3	9.061e-3	3	NC	1	NC	1
570			min	0	10	-.004	1	-.025	2	-1.213e-2	1	NC	1	NC	1
571	M16A	1	max	0	10	0	3	.007	3	3.075e-3	3	NC	1	NC	1
572			min	0	3	-.002	1	-.008	2	-3.736e-3	1	NC	1	NC	1
573		2	max	0	10	-.005	15	.005	1	2.939e-3	3	NC	5	NC	2
574			min	0	3	-.022	4	-.001	10	-3.554e-3	1	4688.414	4	9237.776	1
575		3	max	0	10	-.01	15	.014	1	2.802e-3	3	NC	15	NC	4
576			min	0	3	-.042	4	-.004	3	-3.372e-3	1	2385.774	4	5221.548	1
577		4	max	0	10	-.014	15	.02	1	2.665e-3	3	6963.098	15	NC	4
578			min	0	3	-.061	4	-.008	3	-3.19e-3	1	1636.78	4	3966.897	1
579		5	max	0	10	-.018	15	.024	1	2.529e-3	3	5433.376	15	NC	4
580			min	0	3	-.078	4	-.01	3	-3.008e-3	1	1277.196	4	3421.554	1
581		6	max	0	10	-.022	15	.027	1	2.392e-3	3	4572.76	15	NC	4
582			min	0	3	-.092	4	-.012	3	-2.826e-3	1	1074.896	4	3181.193	1
583		7	max	0	10	-.024	15	.028	1	2.255e-3	3	4055.213	15	NC	4
584			min	0	3	-.104	4	-.012	3	-2.644e-3	1	953.238	4	3118.846	1
585		8	max	0	10	-.026	15	.027	1	2.119e-3	3	3744.607	15	NC	4
586			min	0	3	-.113	4	-.012	3	-2.462e-3	1	880.226	4	3190.666	1
587		9	max	0	10	-.028	15	.026	1	1.982e-3	3	3577.421	15	NC	4
588			min	0	3	-.118	4	-.012	3	-2.28e-3	1	840.926	4	3389.91	1
589		10	max	0	10	-.028	15	.023	1	1.845e-3	3	3524.531	15	NC	4
590			min	0	3	-.12	4	-.011	3	-2.098e-3	1	828.494	4	3736.005	1
591		11	max	0	10	-.028	15	.02	1	1.709e-3	3	3577.421	15	NC	4
592			min	0	3	-.118	4	-.009	3	-1.916e-3	1	840.926	4	4278.458	1
593		12	max	0	10	-.026	15	.017	1	1.572e-3	3	3744.607	15	NC	4
594			min	0	3	-.112	4	-.008	3	-1.734e-3	1	880.226	4	5115.589	1
595		13	max	0	10	-.024	15	.013	1	1.435e-3	3	4055.213	15	NC	3
596			min	0	3	-.104	4	-.006	3	-1.552e-3	1	953.238	4	6442.53	1
597		14	max	0	10	-.022	15	.01	1	1.299e-3	3	4572.76	15	NC	2



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
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Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

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3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



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E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657



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11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



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1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





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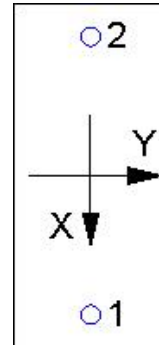
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3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

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8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ec,V} \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ec,V}$	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpg} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpg} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.



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Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
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12. Warnings

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