

Û&@^α ÉQ&É		GE»Áqᵒ₃ ᵀᵀÂ^ā { ǻ/Ö^•ǻ }
PÔX	Ùœᵝ āᵝāÁÚXT ā āÜᵝᵝ ā * Â^•α{	
	Ü^] ^•^} cœā^Ôœ& œā } •ÄÖÜÔÖÄ Æ€	

1. INTRODUCTION

1.1 Project Description

The following sections will cover the determination of forces and structural design calculations for the Schletter, Inc. PVMini ground mount system.

1.2 Construction

Photovoltaic modules are attached to aluminum purlins using clamp fasteners. Purlins are clamped to inclined aluminum girders, which are then connected to aluminum struts. Each support structure is equally spaced.

PV modules are required to meet the following specifications:

	Maximum	Minimum
Height =	1700 mm	1550 mm
Width =	1050 mm	970 mm
Dead Load =	3.00 psf	1.75 psf

Modules Per Row = 1
Module Tilt = 20°
Maximum Height Above Grade = 3 ft

1.3 Technical Codes

- ASCE 7-10 - Chapter 26-31, Wind Loads
- ASCE 7-10 - Chapter 7, Snow Loads
- ASCE 7-10 - Chapter 2, Combination of Loads
- International Building Code, IBC, 2012, 2015
- Aluminum Design Manual, Eighth Edition, 2005

2. LOAD ACTIONS

2.1 Permanent Loads

g_{MAX} =	3.00 psf
g_{MIN} =	1.75 psf



Self-weight of the PV modules.

Typical loading conditions of the module dead loads, snow loads, and wind loads are shown on the left.

2.2 Snow Loads

Ground Snow Load, P_g =	30.00 psf	
Sloped Roof Snow Load, P_s =	20.62 psf	(ASCE 7-10, Eq. 7.4-1)
I_s =	1.00	
C_s =	0.91	
C_e =	0.90	
C_t =	1.20	

2.3 Wind Loads

Design Wind Speed, V =	115 mph	Exposure Category = C
Height \leq	15 ft	Importance Category = II

Peak Velocity Pressure, q_z = 20.76 psf Including the gust factor, $G=0.85$. (ASCE 7-10, Eq. 27.3-1)

Pressure Coefficients

$C_{f+ TOP}$ =	1.05	(Pressure)
$C_{f+ BOTTOM}$ =	1.65	
$C_{f- TOP}$ =	-2.12	(Suction)
$C_{f- BOTTOM}$ =	-1	

Provided pressure coefficients are the result of wind tunnel testing done by Ruscheweyh Consult. Coefficients are located in test report # 1127/0611-1e. Negative forces are applied away from the surface.

2.4 Seismic Loads - N/A

S_S =	0.00	R = 1.25
S_{DS} =	0.00	C_s = 0
S_1 =	0.00	ρ = 1.3
S_{D1} =	0.00	Ω = 1.25
T_a =	0.00	C_d = 1.25

ASCE 7, Section 12.8.1.3: A maximum S_S of 1.5 may be used to calculate the base shear, C_s , of structures under five stories and with a period, T , of 0.5 or less. Therefore, a S_{ds} of 1.0 was used to calculate C_s .

2.5 Combination of Loads

ASCE 7 requires that all structures be checked by specified combinations of loads. Applicable load combinations are provided below.

Strength Design, LRFD

Component stresses are checked using the following LRFD load combinations:

$$\begin{aligned}
 &1.2D + 1.6S + 0.5W \\
 &1.2D + 1.0W + 0.5S \\
 &0.9D + 1.0W^M \\
 &1.54D + 1.3E + 0.2S^R \quad (\text{ASCE 7, Eq 2.3.2-1 through 2.3.2-7}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &0.56D + 1.3E^R \\
 &1.54D + 1.25E + 0.2S^O \\
 &0.56D + 1.25E^O
 \end{aligned}$$

Allowable Stress Design, ASD

Member deflection checks and foundation designs are done according to the following ASD load combinations:

$$\begin{aligned}
 &1.0D + 1.0S \\
 &1.0D + 0.6W \\
 &1.0D + 0.75L + 0.45W + 0.75S \\
 &0.6D + 0.6W^M \quad (\text{ASCE 7, Eq 2.4.1-1 through 2.4.1-8}) \text{ \& (ASCE 7, Section 12.4.3.2)} \\
 &1.238D + 0.875E^O \\
 &1.1785D + 0.65625E + 0.75S^O \\
 &0.362D + 0.875E^O
 \end{aligned}$$

^M Uses the minimum allowable module dead load.

^R Include redundancy factor of 1.3.

^O Includes overstrength factor of 1.25. Used to check seismic drift.

3. STRUCTURAL ANALYSIS

3.1 RISA Results

Appendix B.1 contains outputs from the structural analysis software package, RISA. These outputs are used to accurately determine resultant member and reaction forces from the loads seen throughout Section 2.

3.2 RISA Components

A member and node list has been provided below to correlate the RISA components with the design calculations in Section 4. Items of significance have been listed.

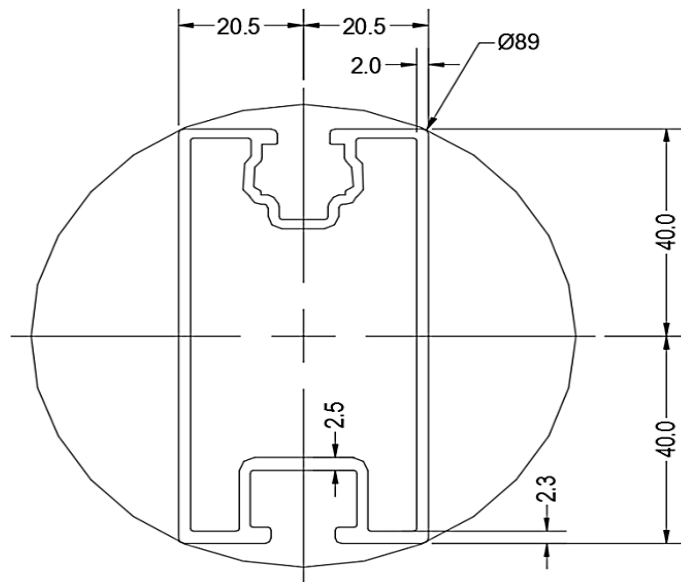
<u>Purlins</u>	<u>Location</u>	<u>Diagonal Struts</u>	<u>Location</u>	<u>Front Reactions</u>	<u>Location</u>
M13	Top	M3	Outer	N7	Outer
M16	Bottom	M7	Inner	N15	Inner
		M11	Outer	N23	Outer
<u>Girders</u>	<u>Location</u>	<u>Rear Struts</u>	<u>Location</u>	<u>Rear Reactions</u>	<u>Location</u>
M1	Outer	M2	Outer	N8	Outer
M5	Inner	M6	Inner	N16	Inner
M9	Outer	M10	Outer	N24	Outer
<u>Front Struts</u>	<u>Location</u>	<u>Bracing</u>			
M4	Outer	M15			
M8	Inner	M16A			
M12	Outer				

4. MEMBER DESIGN CALCULATIONS

4.1 Purlin Design

Aluminum purlins are used to transfer loads to the support structure. Purlins are designed as continuous beams with cantilevers. These are considered beams with internal hinges that can be joined with splices at 25% of the support respective span. See Appendix A.1 for detailed member calculations. Section units are in (mm).

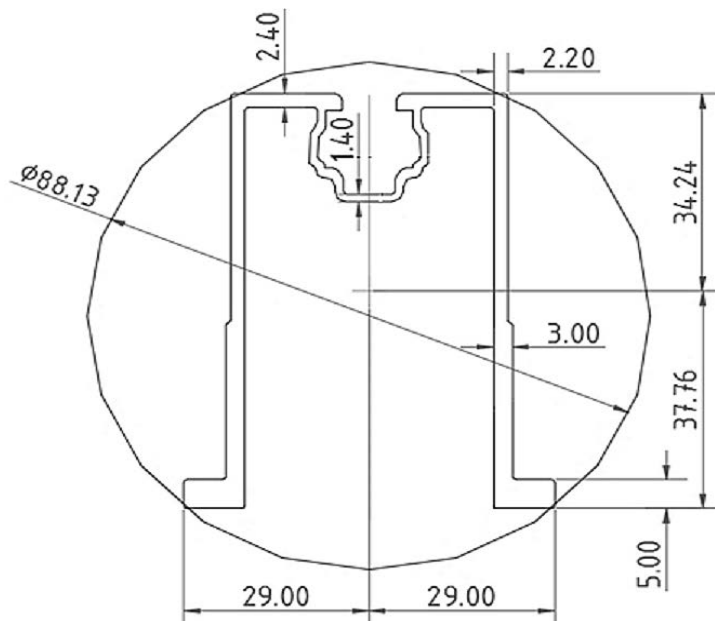
Purlin Type =	ProfiPlusXT
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	93 in
ΦF_{ty} STRONG-AXIS =	28.83 ksi
ΦF_{ty} WEAK-AXIS =	22.71 ksi
S_y =	0.75 in ³
S_x =	0.44 in ³
E =	10100 ksi
I_y =	1.20 in ⁴
I_x =	0.36 in ⁴
A =	0.96 in ²
g =	1.15 lbs/ft
M_y =	1.065 k-ft
M_z =	0.197 k-ft
$M_{y \text{ allowable}}$ =	1.791 k-ft
$M_{z \text{ allowable}}$ =	0.838 k-ft
Utilization =	83%



4.2 Girder Design

Loads from purlins are transferred using an inclined girder, which is connected to a set of aluminum struts. Loads on the girder result from the support reactions of the purlins. See Appendix A.2 for detailed member calculations. Section units are in (mm).

Girder Type =	Flex Profi
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.78 in
ΦF_{ty} AXIAL =	14.29 ksi
ΦF_{ty} STRONG-AXIS =	29.73 ksi
ΦF_{ty} WEAK-AXIS =	13.46 ksi
S_y =	0.59 in ³
S_x =	0.46 in ³
E =	10100 ksi
I_y =	0.88 in ⁴
I_x =	0.52 in ⁴
A =	0.89 in ²
g =	1.07 lbs/ft
M_y =	0.563 k-ft
M_z =	0.000 k-ft
P_n =	0.294 k
$M_{y \text{ allowable}}$ =	1.459 k-ft
$M_{z \text{ allowable}}$ =	0.513 k-ft
$P_{n \text{ allowable}}$ =	12.764 k
Utilization =	41%



4.3 Front Strut Design

The front aluminum strut connects a portion of the girder to the foundation. Vertical girder forces are then transferred down through the strut into the foundation. The strut is attached with single M8 bolts at each end. See Appendix A.3 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	18.00 in
$\Phi F_{ty \text{ AXIAL}}$ =	24.52 ksi
$\Phi F_{ty \text{ BENDING}}$ =	31.19 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.001 k-ft
P_n =	1.510 k
$M_{y \text{ allowable}}$ =	0.423 k-ft
$M_{z \text{ allowable}}$ =	0.423 k-ft
$P_{n \text{ allowable}}$ =	12.310 k
Utilization =	13%



4.4 Diagonal Strut Design

A diagonal aluminum strut braces the support structure. It connects at a front portion of the girder and transfers horizontal forces to the rear foundation connection. The strut is attached with single M8 bolts at each end. See Appendix A.4 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	46.38 in
$\Phi F_{ty \text{ AXIAL}}$ =	7.60 ksi
$\Phi F_{ty \text{ BENDING}}$ =	29.80 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	0.331 k
$M_{y \text{ allowable}}$ =	0.404 k-ft
$M_{z \text{ allowable}}$ =	0.404 k-ft
$P_{n \text{ allowable}}$ =	3.814 k
Utilization =	9%



4.5 Rear Strut Design

An aluminum strut connects the rear portion of the girder to the rear foundation connection. Both vertical and horizontal forces are transferred from the girder. The strut is attached with single M8 bolts at each end. See Appendix A.5 for detailed member calculations. Section units are in (mm).

Strut Type =	30x30x3
Aluminum Type =	6105-T5
F_{ty} =	35 ksi
L_b =	33.07 in
$\Phi F_{ty \text{ AXIAL}}$ =	13.55 ksi
$\Phi F_{ty \text{ BENDING}}$ =	30.37 ksi
S_y =	0.16 in ³
S_x =	0.16 in ³
E =	10100 ksi
I_y =	0.10 in ⁴
I_x =	0.10 in ⁴
A =	0.50 in ²
g =	0.60 lbs/ft
M_y =	0.000 k-ft
M_z =	0.000 k-ft
P_n =	1.225 k
$M_{y \text{ allowable}}$ =	0.411 k-ft
$M_{z \text{ allowable}}$ =	0.411 k-ft
$P_{n \text{ allowable}}$ =	6.803 k
Utilization =	18%



4.6 Cross Brace Design

In order to resist weak side loading, aluminum cross bracing kits are provided. The cross bracing is attached at one end of a rear aluminum strut diagonally down to the bottom end of an adjacent strut. Single M10 bolts are provided at each of the cross bracing. Section units are in (mm).

Brace Type =	1.5x0.25
Aluminum Type =	6061-T6
F_{ty} =	35 ksi
Φ =	0.90
S_y =	0.02 in ³
E =	10100 ksi
I_y =	33.25 in ⁴
A =	0.38 in ²
g =	0.45 lbs/ft
M_y =	0.006 k-ft
P_n =	0.036 k
$M_{y \text{ allowable}}$ =	0.046 k-ft
$P_{n \text{ allowable}}$ =	11.813 k
Utilization =	13%



A cross brace kit is required every 14 bays and is to be installed in centermost bays.

5. FOUNDATION DESIGN CALCULATIONS

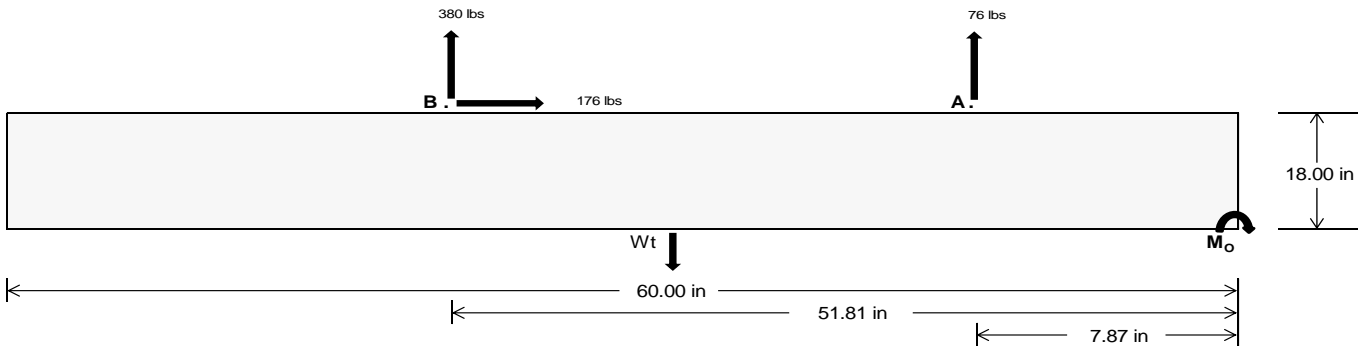
5.1 Helical Pile Foundations

The following LRFD loads include a safety factor of 1.3, and are to be used in conjunction with a Schletter, Inc. Geotechnical Investigation Report. The forces below should fall within the guidelines provided in the Geotechnical Investigation Report. If a Geotechnical Investigation Report is not present, please proceed to Section 5.2 for a concrete foundation design.

	<u>Maximum</u>	<u>Front</u>	<u>Rear</u>
Tensile Load =	338.76	1651.36	k
Compressive Load =	1962.83	1523.45	k
Lateral Load =	4.40	764.86	k
Moment (Weak Axis) =	0.01	0.00	k

5.2 Design of Ballast Foundations

Ballast foundations are used to secure the racking structure in place. The foundations are checked for potential overturning and sliding. Bearing pressures applied by the racking and ballast foundations are checked against the allowable bearing pressures provided by the IBC table 1806.2 (2012, 2015).



Concrete Properties

Weight of Concrete = 145 pcf
Compressive Strength = 2500 psi
Yield Strength = 60000 psi

Overturning Check

$M_o = 23441.2$ in-lbs
Resisting Force Required = 781.37 lbs
S.F. = 1.67
Weight Required = 1302.29 lbs
Minimum Width = 22 in
Weight Provided = 1993.75 lbs

Sliding

Force = 176.37 lbs
Friction = 0.4
Weight Required = 440.94 lbs
Resisting Weight = 1993.75 lbs
Additional Weight Required = 0 lbs

Cohesion

Sliding Force = 176.37 lbs
Cohesion = 130 psf
Area = 9.17 ft²
Resisting = 996.88 lbs
Additional Weight Required = 0 lbs

Shear Key

Additional Force = 0 lbs
Lateral Bearing Pressure = 200 psf/ft
Required Depth = 0.00 ft
 $f'_c = 2500$ psi
Length = 8 in

Footing Reinforcement

Use fiber reinforcing with (1) #5 rebar.

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Use a 60in long x 22in wide x 18in tall ballast foundation to resist sliding. Friction is OK.

Use a 60in long x 22in wide x 18in tall ballast foundation. Cohesion is OK.

Shear key is not required.

Bearing Pressure

Ballast Width
 $P_{ftg} = (145 \text{ pcf})(5 \text{ ft})(1.5 \text{ ft})(1.83 \text{ ft}) =$
22 in 23 in 24 in 25 in
1994 lbs 2084 lbs 2175 lbs 2266 lbs

ASD LC	1.0D + 1.0S				1.0D + 0.6W				1.0D + 0.75L + 0.45W + 0.75S				0.6D + 0.6W			
Width	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in	22 in	23 in	24 in	25 in
F_A	740 lbs	740 lbs	740 lbs	740 lbs	555 lbs	555 lbs	555 lbs	555 lbs	916 lbs	916 lbs	916 lbs	916 lbs	-152 lbs	-152 lbs	-152 lbs	-152 lbs
F_B	541 lbs	541 lbs	541 lbs	541 lbs	489 lbs	489 lbs	489 lbs	489 lbs	731 lbs	731 lbs	731 lbs	731 lbs	-759 lbs	-759 lbs	-759 lbs	-759 lbs
F_V	65 lbs	65 lbs	65 lbs	65 lbs	318 lbs	318 lbs	318 lbs	318 lbs	283 lbs	283 lbs	283 lbs	283 lbs	-353 lbs	-353 lbs	-353 lbs	-353 lbs
P_{total}	3275 lbs	3366 lbs	3456 lbs	3547 lbs	3037 lbs	3128 lbs	3219 lbs	3309 lbs	3641 lbs	3732 lbs	3822 lbs	3913 lbs	285 lbs	339 lbs	394 lbs	448 lbs
M	479 lbs-ft	479 lbs-ft	479 lbs-ft	479 lbs-ft	611 lbs-ft	611 lbs-ft	611 lbs-ft	611 lbs-ft	785 lbs-ft	785 lbs-ft	785 lbs-ft	785 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft	570 lbs-ft
e	0.15 ft	0.14 ft	0.14 ft	0.13 ft	0.20 ft	0.20 ft	0.19 ft	0.18 ft	0.22 ft	0.21 ft	0.21 ft	0.20 ft	2.00 ft	1.68 ft	1.45 ft	1.27 ft
$L/6$	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft	0.83 ft
f_{min}	294.6 psf	291.3 psf	288.2 psf	285.4 psf	251.4 psf	249.9 psf	248.6 psf	247.3 psf	294.5 psf	291.1 psf	288.1 psf	285.3 psf	0.0 psf	0.0 psf	0.0 psf	0.0 psf
f_{max}	419.9 psf	411.1 psf	403.1 psf	395.6 psf	411.3 psf	402.9 psf	395.2 psf	388.1 psf	499.9 psf	487.6 psf	476.4 psf	466.0 psf	207.8 psf	144.0 psf	124.8 psf	116.8 psf

Maximum Bearing Pressure = 500 psf
Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Weak Side Design

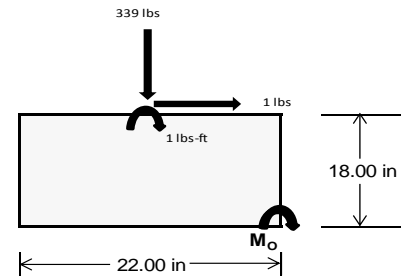
Overturning Check

$M_o = 308.0 \text{ ft-lbs}$
 Resisting Force Required = 336.05 lbs
 S.F. = 1.67
 Weight Required = 560.08 lbs
 Minimum Width = 22 in
 Weight Provided = 1993.75 lbs

A minimum 60in long x 22in wide x 18in tall ballast foundation is required to resist overturning.

Bearing Pressure

ASD LC	1.238D + 0.875E			1.1785D + 0.65625E + 0.75S			0.362D + 0.875E		
Width	22 in			22 in			22 in		
Support	Outer	Inner	Outer	Outer	Inner	Outer	Outer	Inner	Outer
F_v	87 lbs	239 lbs	82 lbs	343 lbs	1044 lbs	339 lbs	25 lbs	70 lbs	24 lbs
F_v	4 lbs	3 lbs	0 lbs	18 lbs	17 lbs	1 lbs	1 lbs	1 lbs	0 lbs
P_{total}	2555 lbs	2707 lbs	2551 lbs	2693 lbs	3393 lbs	2688 lbs	747 lbs	792 lbs	746 lbs
M	6 lbs-ft	5 lbs-ft	0 lbs-ft	31 lbs-ft	25 lbs-ft	2 lbs-ft	2 lbs-ft	2 lbs-ft	0 lbs-ft
e	0.00 ft	0.00 ft	0.00 ft	0.01 ft	0.01 ft	0.00 ft	0.00 ft	0.00 ft	0.00 ft
$L/6$	0.31 ft	1.83 ft	1.83 ft	1.81 ft	1.82 ft	1.83 ft	1.83 ft	1.83 ft	1.83 ft
f_{min}	276.8 sqft	293.5 sqft	278.2 sqft	282.8 sqft	361.1 sqft	292.4 sqft	80.9 sqft	85.8 sqft	81.3 sqft
f_{max}	280.7 psf	297.2 psf	278.4 psf	304.8 psf	379.3 psf	294.1 psf	82.1 psf	86.9 psf	81.4 psf



Maximum Bearing Pressure = 379 psf
 Allowable Bearing Pressure = 1500 psf

Use a 60in long x 22in wide x 18in tall ballast foundation for an acceptable bearing pressure.

Foundation Requirements: 60in long x 22in wide x 18in tall ballast foundation and fiber reinforcing with (1) #5 rebar.

5.3 Foundation Anchors

Threaded rods are anchored to the the ballast foundations using the Simpson AT-XP epoxy solution. LRFD load results are compared to the allowable strengths of the epoxy solution. Please see the supplementary calculations provided by the Simpson Anchor Designer software.

6. DESIGN OF JOINTS AND CONNECTIONS

6.1 Anchorage of Modules to Purlins and Connection of Purlins to Girders

Modules are secured to the purlins with Schletter, Inc. Rapid2+ mounting clamps. Purlins are secured to the girders with the use of a Schletter, Inc. Klicktop connector. The reliability of calculations is uncertain due to limited standards, therefore the strength of the fasteners has been evaluated by load testing.

Fastening of Modules to Purlins

Maximum Uplifting Force =	0.408 k
Allowable Uplift =	1.214 k
Utilization =	<u>34%</u>



Fastening of Purlins to Girders

Maximum Uplifting Force =	1.109 k
Allowable Uplift =	1.116 k
Utilization =	<u>99%</u>



6.2 Bolted Connections

The aluminum struts connect the aluminum girder ends to custom brackets with mounting holes. Cross bracing is attached to rear struts to provide lateral stability. Single M8 bolts are used to attach each end of the strut to the girder and post. ASTM A193/A193M-86 equivalent stainless steel bolts are used.

Front Strut

Maximum Axial Load =	1.510 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>27%</u>

Diagonal Strut

Maximum Axial Load =	0.331 k
M8 Bolt Shear Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>6%</u>



Rear Strut

Maximum Axial Load =	1.225 k
M8 Bolt Capacity =	5.692 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>22%</u>

Bracing

Maximum Axial Load =	0.036 k
M10 Bolt Capacity =	8.894 k
Strut Bearing Capacity =	7.952 k
Utilization =	<u>0%</u>

Bolt and bearing capacities are accounting for double shear (ASCE 8-02, Eq. 5.3.4-1). Struts under compression are shown to demonstrate the load transfer from the girder. Single M8 bolts are located at each end of the strut and are subjected to double shear.

7. SEISMIC DESIGN

7.1 Seismic Drift - N/A

The racking structure has been analyzed under seismic loading. The allowable story drift of the structure must fall within the limits provided by (ASCE 7, Table 12.12-1).

Mean Height, h_{sx} =	29.57 in
Allowable Story Drift for All Other Structures, Δ = {	0.020 h_{sx}
	0.591 in
Max Drift, Δ_{MAX} =	0.047 in
	<u>N/A</u>

The racking structure's reaction to seismic loads is shown to the right. The deflections have been magnified to provide a clear portrayal of potential story drift.



APPENDIX A

A.1 Design of Aluminum Purlins - Aluminum Design Manual, 2005 Edition

Purlin = **ProfiPlus XT**

Strong Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$193.965$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 93.00 \text{ in}$$

$$J = 0.427$$

$$210.771$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 28.6$$

3.4.16

$$b/t = 6.6$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 37.95$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi b [Bp - 1.6Dp \cdot b/t]$$

$$\phi F_L = 22.7 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$\begin{aligned}
 h/t &= 37.95 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 38.1 \\
 m &= 0.63 \\
 C_0 &= 40.784 \\
 Cc &= 39.216 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 79.7 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L St &= 28.8 \text{ ksi} \\
 I_x &= 498305 \text{ mm}^4 \\
 &= 1.197 \text{ in}^4 \\
 y &= 40.784 \text{ mm} \\
 S_x &= 0.746 \text{ in}^3 \\
 M_{\max} St &= 1.791 \text{ k-ft}
 \end{aligned}$$

3.4.18

$$\begin{aligned}
 h/t &= 6.6 \\
 S1 &= \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr} \\
 S1 &= 36.9 \\
 m &= 0.65 \\
 C_0 &= 20.5 \\
 Cc &= 20.5 \\
 S2 &= \frac{k_1 Bbr}{mDbr} \\
 S2 &= 77.3 \\
 \phi F_L &= 1.3\phi y Fcy \\
 \phi F_L &= 43.2 \text{ ksi} \\
 \phi F_L Wk &= 22.7 \text{ ksi} \\
 I_y &= 148662 \text{ mm}^4 \\
 &= 0.357 \text{ in}^4 \\
 x &= 20.5 \text{ mm} \\
 S_y &= 0.443 \text{ in}^3 \\
 M_{\max} Wk &= 0.838 \text{ k-ft}
 \end{aligned}$$

Compression

3.4.9

$$\begin{aligned}
 b/t &= 6.6 \\
 S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\
 S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi} \\
 b/t &= 37.95 \\
 S1 &= 12.21 \\
 S2 &= 32.70 \\
 \phi F_L &= (\phi c k^2 \sqrt{(BpE)}) / (1.6b/t) \\
 \phi F_L &= 21.4 \text{ ksi}
 \end{aligned}$$

3.4.10

$$\begin{aligned}
 Rb/t &= 0.0 \\
 S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\
 S1 &= 6.87 \\
 S2 &= 131.3 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.25 \text{ ksi} \\
 \phi F_L &= 21.42 \text{ ksi} \\
 A &= 620.02 \text{ mm}^2 \\
 &= 0.96 \text{ in}^2 \\
 P_{\max} &= 20.59 \text{ kips}
 \end{aligned}$$

A.2 Design of Aluminum Girders - Aluminum Design Manual, 2005 Edition

Girder = **Flex Profi**

Strong Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &21.5728 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

N/A for Strong Direction

3.4.16

$$\begin{aligned}
 b/t &= 4.29 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 \phi F_L &= \phi y Fcy \\
 \phi F_L &= 33.3 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Strong Direction

Weak Axis:

3.4.11

$$\begin{aligned}
 L_b &= 33.78 \text{ in} \\
 r_y &= 1.374 \\
 C_b &= 1.30 \\
 &24.5845 \\
 S1 &= \frac{1.2(Bc - \frac{\theta_y}{\theta_b} Fcy)}{Dc} \\
 S1 &= 1.37733 \\
 S2 &= 1.2C_c \\
 S2 &= 79.2 \\
 \phi F_L &= \phi b[Bc - Dc * Lb / (1.2 * r_y * \sqrt{(Cb)})] \\
 \phi F_L &= 29.7 \text{ ksi}
 \end{aligned}$$

3.4.15

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{5.1Dp} \\
 S1 &= 3.8 \\
 S2 &= \frac{k_1 Bp}{5.1Dp} \\
 S2 &= 14.7 \\
 F_{UT} &= (\phi b k_2 * \sqrt{(BpE)}) / (5.1b/t) \\
 F_{UT} &= 9.4 \text{ ksi}
 \end{aligned}$$

3.4.16

N/A for Weak Direction

3.4.16

$$\begin{aligned}
 b/t &= 24.46 \\
 S1 &= \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp} \\
 S1 &= 12.2 \\
 S2 &= \frac{k_1 Bp}{1.6Dp} \\
 S2 &= 46.7 \\
 F_{ST} &= \phi b[Bp - 1.6Dp * b/t] \\
 F_{ST} &= 28.2 \text{ ksi}
 \end{aligned}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.2

N/A for Strong Direction

3.4.16.1

N/A for Weak Direction

$$b/t = 24.46$$

$$t = 2.6$$

$$ds = 6.05$$

$$rs = 3.49$$

$$S = 21.70$$

$$\rho_{st} = 0.22$$

$$F_{UT} = 9.37$$

$$F_{ST} = 28.24$$

$$\phi F_L = F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st}$$

$$\phi F_L = 13.5 \text{ ksi}$$

3.4.16.2

3.4.18

$$h/t = 24.46$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 34.4$$

$$m = 0.70$$

$$C_0 = 34.23$$

$$Cc = 37.77$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 72.1$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.7 \text{ ksi}$$

$$I_x = 364470 \text{ mm}^4$$

$$0.876 \text{ in}^4$$

$$y = 37.77 \text{ mm}$$

$$S_x = 0.589 \text{ in}^3$$

$$M_{\max} St = 1.459 \text{ k-ft}$$

3.4.18

$$h/t = 4.29$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 29$$

$$Cc = 29$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 13.5 \text{ ksi}$$

$$I_y = 217168 \text{ mm}^4$$

$$0.522 \text{ in}^4$$

$$x = 29 \text{ mm}$$

$$S_y = 0.457 \text{ in}^3$$

$$M_{\max} Wk = 0.513 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.46067$$

$$r = 1.374 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.90326$$

$$\phi F_L = \phi_{cc} (Bc - Dc^* \lambda)$$

$$\phi F_L = 30.1251 \text{ ksi}$$

3.4.8

$$\begin{aligned} b/t &= 24.46 \\ S1 &= 3.83 \\ S2 &= 10.30 \\ \phi F_L &= (\phi c k^2 \sqrt{(B p E)}) / (5.1 b/t) \\ \phi F_L &= 10.4 \text{ ksi} \end{aligned}$$

3.4.9

$$\begin{aligned} b/t &= 4.29 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.3 \text{ ksi} \\ b/t &= 24.46 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi F_L &= \phi_c [B p - 1.6 D p^* b/t] \\ \phi F_L &= 28.2 \text{ ksi} \end{aligned}$$

3.4.9.1

$$\begin{aligned} b/t &= 24.46 \\ t &= 2.6 \\ d_s &= 6.05 \\ r_s &= 3.49 \\ S &= 21.70 \\ \rho_{st} &= 0.22 \\ F_{UT} &= 10.43 \\ F_{ST} &= 28.24 \\ \phi F_L &= F_{ut} + (F_{st} - F_{ut}) \rho_{st} < F_{st} \\ \phi F_L &= 14.3 \text{ ksi} \end{aligned}$$

3.4.10

$$\begin{aligned} R b/t &= 0.0 \\ S1 &= \left(\frac{B t - \frac{\theta_y}{\theta_b} F_{cy}}{D t} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi F_L &= \phi_y F_{cy} \\ \phi F_L &= 33.25 \text{ ksi} \\ \phi F_L &= 14.29 \text{ ksi} \\ A &= 576.21 \text{ mm}^2 \\ &= 0.89 \text{ in}^2 \\ P_{\max} &= 12.76 \text{ kips} \end{aligned}$$

A.3 Design of Aluminum Struts (Front) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 18.00 \text{ in}$$

$$J = 0.16$$

$$47.2194$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c) / (C_b \sqrt{(I_y J) / 2}))}]$$

$$\phi F_L = 31.2$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 31.2 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.423 \text{ k-ft}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L Wk = 31.2 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.423 \text{ k-ft}$$

Compression

3.4.7

$$\lambda = 0.77182$$

$$r = 0.437 \text{ in}$$

$$S1^* = \frac{Bc - Fcy}{1.6Dc^*}$$

$$S1^* = 0.33515$$

$$S2^* = \frac{Cc}{\pi} \sqrt{Fcy/E}$$

$$S2^* = 1.23671$$

$$\phi_{cc} = 0.83792$$

$$\phi_{FL} = \phi_{cc}(Bc - Dc^*\lambda)$$

$$\phi_{FL} = 24.5226 \text{ ksi}$$

3.4.9

$$b/t = 7.75$$

$$S1 = 12.21 \text{ (See 3.4.16 above for formula)}$$

$$S2 = 32.70 \text{ (See 3.4.16 above for formula)}$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

$$b/t = 7.75$$

$$S1 = 12.21$$

$$S2 = 32.70$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.3 \text{ ksi}$$

3.4.10

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - \frac{\theta_y}{\theta_h} Fcy}{Dt} \right)^2$$

$$S1 = 6.87$$

$$S2 = 131.3$$

$$\phi_{FL} = \phi_y F_{cy}$$

$$\phi_{FL} = 33.25 \text{ ksi}$$

$$\phi_{FL} = 24.52 \text{ ksi}$$

$$A = 323.87 \text{ mm}^2$$

$$0.50 \text{ in}^2$$

$$P_{\max} = 12.31 \text{ kips}$$

A.4 Design of Aluminum Struts (Diagonal) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 46.38 \text{ in}$$

$$J = 0.16$$

$$121.663$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} Fcy}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((LbSc)/(Cb \sqrt{(IyJ)/2}))}]$$

$$\phi F_L = 29.8$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} Fcy}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi y Fcy$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} Fcy}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi y Fcy$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3Fcy}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi y Fcy$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 29.8 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.404 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.98863 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.85841 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 7.59722 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 7.60 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 3.81 \text{ kips}\end{aligned}$$

A.5 Design of Aluminum Struts (Rear) - Aluminum Design Manual, 2005 Edition

Strut = **30x30x3**

Strong Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4 \text{ ksi}$$

Weak Axis:

3.4.14

$$L_b = 33.07 \text{ in}$$

$$J = 0.16$$

$$86.7548$$

$$S1 = \left(\frac{Bc - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dc} \right)^2$$

$$S1 = 0.51461$$

$$S2 = \left(\frac{C_c}{1.6} \right)^2$$

$$S2 = 1701.56$$

$$\phi F_L = \phi b [Bc - 1.6Dc \sqrt{((L_b S_c)/(C_b \sqrt{(I_y J)/2}))}]$$

$$\phi F_L = 30.4$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16

$$b/t = 7.75$$

$$S1 = \frac{Bp - \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dp}$$

$$S1 = 12.2$$

$$S2 = \frac{k_1 Bp}{1.6Dp}$$

$$S2 = 46.7$$

$$\phi F_L = \phi_y F_{cy}$$

$$\phi F_L = 33.3 \text{ ksi}$$

3.4.16.1 Not Used

$$Rb/t = 0.0$$

$$S1 = \left(\frac{Bt - 1.17 \frac{\theta_y}{\theta_b} F_{cy}}{1.6Dt} \right)^2$$

$$S1 = 1.1$$

$$S2 = C_t$$

$$S2 = 141.0$$

$$\phi F_L = 1.17 \phi_y F_{cy}$$

$$\phi F_L = 38.9 \text{ ksi}$$

3.4.16.1

N/A for Weak Direction

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

3.4.18

$$h/t = 7.75$$

$$S1 = \frac{Bbr - \frac{\theta_y}{\theta_b} 1.3F_{cy}}{mDbr}$$

$$S1 = 36.9$$

$$m = 0.65$$

$$C_0 = 15$$

$$Cc = 15$$

$$S2 = \frac{k_1 Bbr}{mDbr}$$

$$S2 = 77.3$$

$$\phi F_L = 1.3 \phi_y F_{cy}$$

$$\phi F_L = 43.2 \text{ ksi}$$

$$\phi F_L St = 30.4 \text{ ksi}$$

$$I_x = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$y = 15 \text{ mm}$$

$$S_x = 0.163 \text{ in}^3$$

$$M_{\max} St = 0.411 \text{ k-ft}$$

$$\phi F_L Wk = 33.3 \text{ ksi}$$

$$I_y = 39958.2 \text{ mm}^4$$

$$0.096 \text{ in}^4$$

$$x = 15 \text{ mm}$$

$$S_y = 0.163 \text{ in}^3$$

$$M_{\max} Wk = 0.450 \text{ k-ft}$$

Compression

3.4.7

$$\begin{aligned}\lambda &= 1.41804 \\ r &= 0.437 \text{ in} \\ S1^* &= \frac{Bc - Fcy}{1.6Dc^*} \\ S1^* &= 0.33515 \\ S2^* &= \frac{Cc}{\pi} \sqrt{Fcy/E} \\ S2^* &= 1.23671 \\ \phi_{cc} &= 0.77853 \\ \phi_{FL} &= (\phi_{cc} Fcy)/(\lambda^2) \\ \phi_{FL} &= 13.5508 \text{ ksi}\end{aligned}$$

3.4.9

$$\begin{aligned}b/t &= 7.75 \\ S1 &= 12.21 \text{ (See 3.4.16 above for formula)} \\ S2 &= 32.70 \text{ (See 3.4.16 above for formula)} \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi} \\ b/t &= 7.75 \\ S1 &= 12.21 \\ S2 &= 32.70 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.3 \text{ ksi}\end{aligned}$$

3.4.10

$$\begin{aligned}Rb/t &= 0.0 \\ S1 &= \left(\frac{Bt - \frac{\theta_y}{\theta_b} Fcy}{Dt} \right)^2 \\ S1 &= 6.87 \\ S2 &= 131.3 \\ \phi_{FL} &= \phi_y Fcy \\ \phi_{FL} &= 33.25 \text{ ksi} \\ \phi_{FL} &= 13.55 \text{ ksi} \\ A &= 323.87 \text{ mm}^2 \\ &= 0.50 \text{ in}^2 \\ P_{max} &= 6.80 \text{ kips}\end{aligned}$$

APPENDIX B

B.1

The following pages will contain the results from RISA. Please refer back to Section 2 for load information and Section 4-5 for member and foundation design.



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Joint Reactions

	Joint		X [lb]	LC	Y [lb]	LC	Z [lb]	LC	MX [k-ft]	LC	MY [k-ft]	LC	MZ [k-ft]	LC
1	N8	max	138.276	2	322.151	1	0	5	0	1	0	1	0	1
2		min	-184.518	3	-387.256	3	-.128	1	0	3	0	1	0	1
3	N7	max	0	15	527.602	1	-.058	15	0	15	0	1	0	1
4		min	-.184	1	-70.784	3	-1.497	1	-.003	1	0	1	0	1
5	N15	max	0	15	1509.873	1	.579	1	.001	1	0	1	0	1
6		min	-1.917	1	-260.584	3	-.285	3	0	3	0	1	0	1
7	N16	max	553.88	2	1171.888	1	-.183	10	0	1	0	1	0	1
8		min	-588.35	3	-1270.279	3	-33.163	3	0	3	0	1	0	1
9	N23	max	0	15	527.371	1	3.385	1	.006	1	0	1	0	1
10		min	-.184	1	-70.349	3	.123	15	0	15	0	1	0	1
11	N24	max	138.742	2	327.591	1	33.408	3	.002	1	0	1	0	1
12		min	-184.595	3	-384.491	3	.033	10	0	3	0	1	0	1
13	Totals:	max	828.844	2	4386.477	1	0	1						
14		min	-957.772	3	-2443.744	3	0	12						

Envelope Member Section Forces

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
1	M2	1	max	373.974	1	.644	4	.728	1	0	15	0	3	0	1
2			min	-366.873	3	.152	15	-.046	3	-.001	1	0	1	0	1
3		2	max	374.08	1	.602	4	.728	1	0	15	0	1	0	15
4			min	-366.793	3	.143	15	-.046	3	-.001	1	0	10	0	4
5		3	max	374.187	1	.561	4	.728	1	0	15	0	1	0	15
6			min	-366.713	3	.133	15	-.046	3	-.001	1	0	3	0	4
7		4	max	374.293	1	.52	4	.728	1	0	15	0	1	0	15
8			min	-366.634	3	.123	15	-.046	3	-.001	1	0	3	0	4
9		5	max	374.4	1	.478	4	.728	1	0	15	0	1	0	15
10			min	-366.554	3	.113	15	-.046	3	-.001	1	0	3	0	4
11		6	max	374.506	1	.437	4	.728	1	0	15	0	1	0	15
12			min	-366.474	3	.104	15	-.046	3	-.001	1	0	3	0	4
13		7	max	374.613	1	.396	4	.728	1	0	15	0	1	0	15
14			min	-366.394	3	.094	15	-.046	3	-.001	1	0	3	0	4
15		8	max	374.719	1	.355	4	.728	1	0	15	0	1	0	15
16			min	-366.314	3	.084	15	-.046	3	-.001	1	0	3	0	4
17		9	max	374.826	1	.313	4	.728	1	0	15	0	1	0	15
18			min	-366.234	3	.075	15	-.046	3	-.001	1	0	3	0	4
19		10	max	374.932	1	.272	4	.728	1	0	15	0	1	0	15
20			min	-366.154	3	.065	15	-.046	3	-.001	1	0	3	0	4
21		11	max	375.039	1	.231	4	.728	1	0	15	.001	1	0	15
22			min	-366.074	3	.055	15	-.046	3	-.001	1	0	3	0	4
23		12	max	375.145	1	.19	4	.728	1	0	15	.001	1	0	15
24			min	-365.994	3	.046	15	-.046	3	-.001	1	0	3	0	4
25		13	max	375.252	1	.148	4	.728	1	0	15	.001	1	0	15
26			min	-365.914	3	.036	15	-.046	3	-.001	1	0	3	0	4
27		14	max	375.359	1	.107	4	.728	1	0	15	.001	1	0	15
28			min	-365.834	3	.026	15	-.046	3	-.001	1	0	3	0	4
29		15	max	375.465	1	.073	2	.728	1	0	15	.001	1	0	15
30			min	-365.755	3	.014	12	-.046	3	-.001	1	0	3	0	4
31		16	max	375.572	1	.041	2	.728	1	0	15	.002	1	0	15
32			min	-365.675	3	-.005	3	-.046	3	-.001	1	0	3	0	4
33		17	max	375.678	1	.009	10	.728	1	0	15	.002	1	0	15
34			min	-365.595	3	-.03	1	-.046	3	-.001	1	0	3	0	4
35		18	max	375.785	1	-.013	15	.728	1	0	15	.002	1	0	15
36			min	-365.515	3	-.062	1	-.046	3	-.001	1	0	3	0	4
37		19	max	375.891	1	-.022	15	.728	1	0	15	.002	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
38		min	-365.435	3	-.099	4	-.046	3	-.001	1	0	3	0	4
39	M3	1	max	66.398	2	1.795	4	-.023	15	0	.002	1	0	4
40		min	-98.261	9	.423	15	-.687	1	0	1	0	15	0	15
41		2	max	66.33	2	1.618	4	-.023	15	0	.002	1	0	4
42		min	-98.317	9	.381	15	-.687	1	0	1	0	15	0	15
43		3	max	66.262	2	1.44	4	-.023	15	0	.002	1	0	2
44		min	-98.374	9	.339	15	-.687	1	0	1	0	15	0	3
45		4	max	66.194	2	1.262	4	-.023	15	0	.002	1	0	15
46		min	-98.43	9	.297	15	-.687	1	0	1	0	15	0	4
47		5	max	66.127	2	1.085	4	-.023	15	0	.001	1	0	15
48		min	-98.487	9	.256	15	-.687	1	0	1	0	15	0	4
49		6	max	66.059	2	.907	4	-.023	15	0	.001	1	0	15
50		min	-98.543	9	.214	15	-.687	1	0	1	0	15	0	4
51		7	max	65.991	2	.729	4	-.023	15	0	.001	1	0	15
52		min	-98.6	9	.172	15	-.687	1	0	1	0	15	0	4
53		8	max	65.923	2	.552	4	-.023	15	0	.001	1	0	15
54		min	-98.657	9	.13	15	-.687	1	0	1	0	15	0	4
55		9	max	65.855	2	.374	4	-.023	15	0	0	1	0	15
56		min	-98.713	9	.089	15	-.687	1	0	1	0	15	-.001	4
57		10	max	65.787	2	.196	4	-.023	15	0	0	1	0	15
58		min	-98.77	9	.047	15	-.687	1	0	1	0	15	-.001	4
59		11	max	65.719	2	.03	2	-.023	15	0	0	1	0	15
60		min	-98.826	9	-.003	3	-.687	1	0	1	0	15	-.001	4
61		12	max	65.652	2	-.037	15	-.023	15	0	0	1	0	15
62		min	-98.883	9	-.159	4	-.687	1	0	1	0	15	-.001	4
63		13	max	65.584	2	-.078	15	-.023	15	0	0	1	0	15
64		min	-98.939	9	-.337	4	-.687	1	0	1	0	12	-.001	4
65		14	max	65.516	2	-.12	15	-.023	15	0	0	1	0	15
66		min	-98.996	9	-.514	4	-.687	1	0	1	0	12	-.001	4
67		15	max	65.448	2	-.162	15	-.023	15	0	0	1	0	15
68		min	-99.052	9	-.692	4	-.687	1	0	1	0	3	0	4
69		16	max	65.38	2	-.204	15	-.023	15	0	0	15	0	15
70		min	-99.109	9	-.869	4	-.687	1	0	1	0	1	0	4
71		17	max	65.312	2	-.245	15	-.023	15	0	0	15	0	15
72		min	-99.166	9	-1.047	4	-.687	1	0	1	0	1	0	4
73		18	max	65.244	2	-.287	15	-.023	15	0	0	15	0	15
74		min	-99.222	9	-1.225	4	-.687	1	0	1	0	1	0	4
75		19	max	65.176	2	-.329	15	-.023	15	0	0	15	0	1
76		min	-99.279	9	-1.402	4	-.687	1	0	1	0	1	0	1
77	M4	1	max	526.437	1	0	1	-.058	15	0	0	3	0	1
78		min	-71.658	3	0	1	-1.641	1	0	1	0	1	0	1
79		2	max	526.502	1	0	1	-.058	15	0	0	15	0	1
80		min	-71.609	3	0	1	-1.641	1	0	1	0	1	0	1
81		3	max	526.567	1	0	1	-.058	15	0	0	15	0	1
82		min	-71.561	3	0	1	-1.641	1	0	1	0	1	0	1
83		4	max	526.632	1	0	1	-.058	15	0	0	15	0	1
84		min	-71.512	3	0	1	-1.641	1	0	1	0	1	0	1
85		5	max	526.696	1	0	1	-.058	15	0	0	15	0	1
86		min	-71.464	3	0	1	-1.641	1	0	1	0	1	0	1
87		6	max	526.761	1	0	1	-.058	15	0	0	15	0	1
88		min	-71.415	3	0	1	-1.641	1	0	1	0	1	0	1
89		7	max	526.826	1	0	1	-.058	15	0	0	15	0	1
90		min	-71.366	3	0	1	-1.641	1	0	1	0	1	0	1
91		8	max	526.89	1	0	1	-.058	15	0	0	15	0	1
92		min	-71.318	3	0	1	-1.641	1	0	1	-.001	1	0	1
93		9	max	526.955	1	0	1	-.058	15	0	0	15	0	1
94		min	-71.269	3	0	1	-1.641	1	0	1	-.001	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
95	10	max	527.02	1	0	1	-.058	15	0	1	0	15	0	1
96		min	-71.221	3	0	1	-1.641	1	0	1	-.001	1	0	1
97	11	max	527.085	1	0	1	-.058	15	0	1	0	15	0	1
98		min	-71.172	3	0	1	-1.641	1	0	1	-.001	1	0	1
99	12	max	527.149	1	0	1	-.058	15	0	1	0	15	0	1
100		min	-71.124	3	0	1	-1.641	1	0	1	-.002	1	0	1
101	13	max	527.214	1	0	1	-.058	15	0	1	0	15	0	1
102		min	-71.075	3	0	1	-1.641	1	0	1	-.002	1	0	1
103	14	max	527.279	1	0	1	-.058	15	0	1	0	15	0	1
104		min	-71.027	3	0	1	-1.641	1	0	1	-.002	1	0	1
105	15	max	527.343	1	0	1	-.058	15	0	1	0	15	0	1
106		min	-70.978	3	0	1	-1.641	1	0	1	-.002	1	0	1
107	16	max	527.408	1	0	1	-.058	15	0	1	0	15	0	1
108		min	-70.93	3	0	1	-1.641	1	0	1	-.002	1	0	1
109	17	max	527.473	1	0	1	-.058	15	0	1	0	15	0	1
110		min	-70.881	3	0	1	-1.641	1	0	1	-.002	1	0	1
111	18	max	527.537	1	0	1	-.058	15	0	1	0	15	0	1
112		min	-70.833	3	0	1	-1.641	1	0	1	-.003	1	0	1
113	19	max	527.602	1	0	1	-.058	15	0	1	0	15	0	1
114		min	-70.784	3	0	1	-1.641	1	0	1	-.003	1	0	1
115	M6	1	max	1223.049	1	.638	.264	1	0	1	0	3	0	1
116		min	-1197.667	3	.151	15	-.127	3	0	15	0	1	0	1
117	2	max	1223.155	1	.596	4	.264	1	0	1	0	3	0	15
118		min	-1197.587	3	.142	15	-.127	3	0	15	0	11	0	4
119	3	max	1223.262	1	.555	4	.264	1	0	1	0	2	0	15
120		min	-1197.507	3	.132	15	-.127	3	0	15	0	15	0	4
121	4	max	1223.368	1	.514	4	.264	1	0	1	0	1	0	15
122		min	-1197.427	3	.122	15	-.127	3	0	15	0	12	0	4
123	5	max	1223.475	1	.473	4	.264	1	0	1	0	1	0	15
124		min	-1197.347	3	.113	15	-.127	3	0	15	0	3	0	4
125	6	max	1223.581	1	.431	4	.264	1	0	1	0	1	0	15
126		min	-1197.267	3	.103	15	-.127	3	0	15	0	3	0	4
127	7	max	1223.688	1	.39	4	.264	1	0	1	0	1	0	15
128		min	-1197.187	3	.093	15	-.127	3	0	15	0	3	0	4
129	8	max	1223.794	1	.349	4	.264	1	0	1	0	1	0	15
130		min	-1197.107	3	.084	15	-.127	3	0	15	0	3	0	4
131	9	max	1223.901	1	.316	2	.264	1	0	1	0	1	0	15
132		min	-1197.028	3	.074	15	-.127	3	0	15	0	3	0	4
133	10	max	1224.007	1	.284	2	.264	1	0	1	0	1	0	15
134		min	-1196.948	3	.064	15	-.127	3	0	15	0	3	0	4
135	11	max	1224.114	1	.252	2	.264	1	0	1	0	1	0	15
136		min	-1196.868	3	.052	12	-.127	3	0	15	0	3	0	4
137	12	max	1224.221	1	.219	2	.264	1	0	1	0	1	0	15
138		min	-1196.788	3	.036	12	-.127	3	0	15	0	3	0	4
139	13	max	1224.327	1	.187	2	.264	1	0	1	0	1	0	15
140		min	-1196.708	3	.02	12	-.127	3	0	15	0	3	0	4
141	14	max	1224.434	1	.155	2	.264	1	0	1	0	1	0	15
142		min	-1196.628	3	0	3	-.127	3	0	15	0	3	0	4
143	15	max	1224.54	1	.123	2	.264	1	0	1	0	1	0	15
144		min	-1196.548	3	-.025	3	-.127	3	0	15	0	3	0	4
145	16	max	1224.647	1	.091	2	.264	1	0	1	0	1	0	15
146		min	-1196.468	3	-.049	3	-.127	3	0	15	0	3	0	2
147	17	max	1224.753	1	.059	2	.264	1	0	1	0	1	0	15
148		min	-1196.388	3	-.073	3	-.127	3	0	15	0	3	0	2
149	18	max	1224.86	1	.026	2	.264	1	0	1	0	1	0	15
150		min	-1196.308	3	-.097	3	-.127	3	0	15	0	3	0	2
151	19	max	1224.966	1	-.006	2	.264	1	0	1	0	1	0	15



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
152		min	-1196.228	3	-.121	3	-.127	3	0	15	0	3	0	2
153	M7	1	max	330.663	2	1.794	4	.014	1	0	2	0	2	2
154		min	-265.128	3	.423	15	-.003	10	0	3	0	3	0	12
155		2	max	330.595	2	1.617	4	.014	1	0	2	0	2	2
156		min	-265.179	3	.381	15	-.003	10	0	3	0	3	0	3
157		3	max	330.528	2	1.439	4	.014	1	0	2	0	2	2
158		min	-265.229	3	.339	15	-.003	10	0	3	0	3	0	3
159		4	max	330.46	2	1.261	4	.014	1	0	2	0	2	2
160		min	-265.28	3	.297	15	-.003	10	0	3	0	3	0	3
161		5	max	330.392	2	1.084	4	.014	1	0	2	0	2	15
162		min	-265.331	3	.256	15	-.003	10	0	3	0	3	0	4
163		6	max	330.324	2	.906	4	.014	1	0	2	0	2	15
164		min	-265.382	3	.214	15	-.003	10	0	3	0	3	0	4
165		7	max	330.256	2	.728	4	.014	1	0	2	0	2	15
166		min	-265.433	3	.172	15	-.003	10	0	3	0	3	0	4
167		8	max	330.188	2	.551	4	.014	1	0	2	0	2	15
168		min	-265.484	3	.13	15	-.003	10	0	3	0	3	-.001	4
169		9	max	330.12	2	.373	4	.014	1	0	2	0	2	15
170		min	-265.535	3	.089	15	-.003	10	0	3	0	3	-.001	4
171		10	max	330.052	2	.217	2	.014	1	0	2	0	2	15
172		min	-265.586	3	.043	12	-.003	10	0	3	0	3	-.001	4
173		11	max	329.985	2	.078	2	.014	1	0	2	0	2	15
174		min	-265.637	3	-.045	3	-.003	10	0	3	0	3	-.001	4
175		12	max	329.917	2	-.037	15	.014	1	0	2	0	2	15
176		min	-265.688	3	-.16	4	-.003	10	0	3	0	3	-.001	4
177		13	max	329.849	2	-.078	15	.014	1	0	2	0	2	15
178		min	-265.738	3	-.338	4	-.003	10	0	3	0	3	-.001	4
179		14	max	329.781	2	-.12	15	.014	1	0	2	0	2	15
180		min	-265.789	3	-.515	4	-.003	10	0	3	0	3	-.001	4
181		15	max	329.713	2	-.162	15	.014	1	0	2	0	2	15
182		min	-265.84	3	-.693	4	-.003	10	0	3	0	3	0	4
183		16	max	329.645	2	-.204	15	.014	1	0	2	0	2	15
184		min	-265.891	3	-.871	4	-.003	10	0	3	0	3	0	4
185		17	max	329.577	2	-.246	15	.014	1	0	2	0	2	15
186		min	-265.942	3	-1.048	4	-.003	10	0	3	0	3	0	4
187		18	max	329.51	2	-.287	15	.014	1	0	2	0	2	15
188		min	-265.993	3	-1.226	4	-.003	10	0	3	0	3	0	4
189		19	max	329.442	2	-.329	15	.014	1	0	2	0	2	1
190		min	-266.044	3	-1.404	4	-.003	10	0	3	0	3	0	1
191	M8	1	max	1508.708	1	0	1	.768	1	0	1	0	15	0
192		min	-261.458	3	0	1	-.273	3	0	1	0	1	0	1
193		2	max	1508.773	1	0	1	.768	1	0	1	0	1	0
194		min	-261.409	3	0	1	-.273	3	0	1	0	3	0	1
195		3	max	1508.838	1	0	1	.768	1	0	1	0	1	0
196		min	-261.361	3	0	1	-.273	3	0	1	0	3	0	1
197		4	max	1508.902	1	0	1	.768	1	0	1	0	1	0
198		min	-261.312	3	0	1	-.273	3	0	1	0	3	0	1
199		5	max	1508.967	1	0	1	.768	1	0	1	0	1	0
200		min	-261.264	3	0	1	-.273	3	0	1	0	3	0	1
201		6	max	1509.032	1	0	1	.768	1	0	1	0	1	0
202		min	-261.215	3	0	1	-.273	3	0	1	0	3	0	1
203		7	max	1509.097	1	0	1	.768	1	0	1	0	1	0
204		min	-261.166	3	0	1	-.273	3	0	1	0	3	0	1
205		8	max	1509.161	1	0	1	.768	1	0	1	0	1	0
206		min	-261.118	3	0	1	-.273	3	0	1	0	3	0	1
207		9	max	1509.226	1	0	1	.768	1	0	1	0	1	0
208		min	-261.069	3	0	1	-.273	3	0	1	0	3	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
209	10	max	1509.291	1	0	1	.768	1	0	1	0	1	0	1
210		min	-261.021	3	0	1	-.273	3	0	1	0	3	0	1
211	11	max	1509.355	1	0	1	.768	1	0	1	0	1	0	1
212		min	-260.972	3	0	1	-.273	3	0	1	0	3	0	1
213	12	max	1509.42	1	0	1	.768	1	0	1	0	1	0	1
214		min	-260.924	3	0	1	-.273	3	0	1	0	3	0	1
215	13	max	1509.485	1	0	1	.768	1	0	1	0	1	0	1
216		min	-260.875	3	0	1	-.273	3	0	1	0	3	0	1
217	14	max	1509.55	1	0	1	.768	1	0	1	0	1	0	1
218		min	-260.827	3	0	1	-.273	3	0	1	0	3	0	1
219	15	max	1509.614	1	0	1	.768	1	0	1	0	1	0	1
220		min	-260.778	3	0	1	-.273	3	0	1	0	3	0	1
221	16	max	1509.679	1	0	1	.768	1	0	1	.001	1	0	1
222		min	-260.73	3	0	1	-.273	3	0	1	0	3	0	1
223	17	max	1509.744	1	0	1	.768	1	0	1	.001	1	0	1
224		min	-260.681	3	0	1	-.273	3	0	1	0	3	0	1
225	18	max	1509.808	1	0	1	.768	1	0	1	.001	1	0	1
226		min	-260.633	3	0	1	-.273	3	0	1	0	3	0	1
227	19	max	1509.873	1	0	1	.768	1	0	1	.001	1	0	1
228		min	-260.584	3	0	1	-.273	3	0	1	0	3	0	1
229	M10	1	max	385.402	1	.634	4	-.005	15	.001	1	0	1	0
230		min	-353.465	3	.151	15	-.152	1	0	3	0	3	0	1
231	2	max	385.509	1	.593	4	-.005	15	.001	1	0	1	0	15
232		min	-353.385	3	.141	15	-.152	1	0	3	0	3	0	4
233	3	max	385.615	1	.551	4	-.005	15	.001	1	0	1	0	15
234		min	-353.305	3	.132	15	-.152	1	0	3	0	3	0	4
235	4	max	385.722	1	.51	4	-.005	15	.001	1	0	1	0	15
236		min	-353.225	3	.122	15	-.152	1	0	3	0	3	0	4
237	5	max	385.828	1	.469	4	-.005	15	.001	1	0	1	0	15
238		min	-353.145	3	.112	15	-.152	1	0	3	0	3	0	4
239	6	max	385.935	1	.428	4	-.005	15	.001	1	0	2	0	15
240		min	-353.066	3	.102	15	-.152	1	0	3	0	3	0	4
241	7	max	386.041	1	.386	4	-.005	15	.001	1	0	2	0	15
242		min	-352.986	3	.093	15	-.152	1	0	3	0	3	0	4
243	8	max	386.148	1	.345	4	-.005	15	.001	1	0	15	0	15
244		min	-352.906	3	.083	15	-.152	1	0	3	0	3	0	4
245	9	max	386.255	1	.304	4	-.005	15	.001	1	0	15	0	15
246		min	-352.826	3	.073	15	-.152	1	0	3	0	3	0	4
247	10	max	386.361	1	.263	4	-.005	15	.001	1	0	15	0	15
248		min	-352.746	3	.064	15	-.152	1	0	3	0	3	0	4
249	11	max	386.468	1	.221	4	-.005	15	.001	1	0	15	0	15
250		min	-352.666	3	.054	15	-.152	1	0	3	0	1	0	4
251	12	max	386.574	1	.18	4	-.005	15	.001	1	0	15	0	15
252		min	-352.586	3	.044	15	-.152	1	0	3	0	1	0	4
253	13	max	386.681	1	.139	4	-.005	15	.001	1	0	15	0	15
254		min	-352.506	3	.035	15	-.152	1	0	3	0	1	0	4
255	14	max	386.787	1	.105	2	-.005	15	.001	1	0	15	0	15
256		min	-352.426	3	.019	1	-.152	1	0	3	0	1	0	4
257	15	max	386.894	1	.073	2	-.005	15	.001	1	0	15	0	15
258		min	-352.346	3	-.013	1	-.152	1	0	3	0	1	0	4
259	16	max	387	1	.041	2	-.005	15	.001	1	0	15	0	15
260		min	-352.266	3	-.045	1	-.152	1	0	3	0	1	0	4
261	17	max	387.107	1	.009	10	-.005	15	.001	1	0	15	0	15
262		min	-352.187	3	-.077	1	-.152	1	0	3	0	1	0	4
263	18	max	387.213	1	-.014	12	-.005	15	.001	1	0	15	0	15
264		min	-352.107	3	-.11	1	-.152	1	0	3	0	1	0	4
265	19	max	387.32	1	-.024	15	-.005	15	.001	1	0	15	0	15

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
266			min	-352.027	3	-.142	1	-.152	1	0	3	0	1	0	4
267	M11	1	max	66.1	2	1.8	4	.805	1	.001	1	0	3	0	4
268			min	-98.158	9	.423	15	.016	12	0	15	-.002	1	0	15
269		2	max	66.033	2	1.622	4	.805	1	.001	1	0	3	0	4
270			min	-98.215	9	.382	15	.016	12	0	15	-.002	1	0	12
271		3	max	65.965	2	1.445	4	.805	1	.001	1	0	3	0	2
272			min	-98.271	9	.34	15	.016	12	0	15	-.002	1	0	3
273		4	max	65.897	2	1.267	4	.805	1	.001	1	0	3	0	15
274			min	-98.328	9	.298	15	.016	12	0	15	-.002	1	0	3
275		5	max	65.829	2	1.09	4	.805	1	.001	1	0	3	0	15
276			min	-98.384	9	.256	15	.016	12	0	15	-.001	1	0	4
277		6	max	65.761	2	.912	4	.805	1	.001	1	0	3	0	15
278			min	-98.441	9	.215	15	.016	12	0	15	-.001	1	0	4
279		7	max	65.693	2	.734	4	.805	1	.001	1	0	3	0	15
280			min	-98.497	9	.173	15	.016	12	0	15	-.001	1	0	4
281		8	max	65.625	2	.557	4	.805	1	.001	1	0	3	0	15
282			min	-98.554	9	.131	15	.016	12	0	15	0	1	0	4
283		9	max	65.558	2	.379	4	.805	1	.001	1	0	3	0	15
284			min	-98.61	9	.089	15	.016	12	0	15	0	1	-.001	4
285		10	max	65.49	2	.201	4	.805	1	.001	1	0	3	0	15
286			min	-98.667	9	.048	15	.016	12	0	15	0	1	-.001	4
287		11	max	65.422	2	.03	2	.805	1	.001	1	0	3	0	15
288			min	-98.724	9	-.021	3	.016	12	0	15	0	1	-.001	4
289		12	max	65.354	2	-.036	15	.805	1	.001	1	0	3	0	15
290			min	-98.78	9	-.154	4	.016	12	0	15	0	1	-.001	4
291		13	max	65.286	2	-.078	15	.805	1	.001	1	0	3	0	15
292			min	-98.837	9	-.332	4	.016	12	0	15	0	2	-.001	4
293		14	max	65.218	2	-.119	15	.805	1	.001	1	0	3	0	15
294			min	-98.893	9	-.509	4	.016	12	0	15	0	10	-.001	4
295		15	max	65.15	2	-.161	15	.805	1	.001	1	0	1	0	15
296			min	-98.95	9	-.687	4	.016	12	0	15	0	15	0	4
297		16	max	65.083	2	-.203	15	.805	1	.001	1	0	1	0	15
298			min	-99.006	9	-.865	4	.016	12	0	15	0	15	0	4
299		17	max	65.015	2	-.245	15	.805	1	.001	1	0	1	0	15
300			min	-99.063	9	-1.042	4	.016	12	0	15	0	15	0	4
301		18	max	64.947	2	-.286	15	.805	1	.001	1	0	1	0	15
302			min	-99.119	9	-1.22	4	.016	12	0	15	0	15	0	4
303		19	max	64.879	2	-.328	15	.805	1	.001	1	.001	1	0	1
304			min	-99.176	9	-1.398	4	.016	12	0	15	0	15	0	1
305	M12	1	max	526.206	1	0	1	3.707	1	0	1	0	1	0	1
306			min	-71.223	3	0	1	.123	15	0	1	0	3	0	1
307		2	max	526.271	1	0	1	3.707	1	0	1	0	1	0	1
308			min	-71.174	3	0	1	.123	15	0	1	0	15	0	1
309		3	max	526.335	1	0	1	3.707	1	0	1	0	1	0	1
310			min	-71.126	3	0	1	.123	15	0	1	0	15	0	1
311		4	max	526.4	1	0	1	3.707	1	0	1	.001	1	0	1
312			min	-71.077	3	0	1	.123	15	0	1	0	15	0	1
313		5	max	526.465	1	0	1	3.707	1	0	1	.001	1	0	1
314			min	-71.029	3	0	1	.123	15	0	1	0	15	0	1
315		6	max	526.53	1	0	1	3.707	1	0	1	.002	1	0	1
316			min	-70.98	3	0	1	.123	15	0	1	0	15	0	1
317		7	max	526.594	1	0	1	3.707	1	0	1	.002	1	0	1
318			min	-70.932	3	0	1	.123	15	0	1	0	15	0	1
319		8	max	526.659	1	0	1	3.707	1	0	1	.002	1	0	1
320			min	-70.883	3	0	1	.123	15	0	1	0	15	0	1
321		9	max	526.724	1	0	1	3.707	1	0	1	.003	1	0	1
322			min	-70.835	3	0	1	.123	15	0	1	0	15	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
323	10	max	526.788	1	0	1	3.707	1	0	1	.003	1	0	1
324		min	-70.786	3	0	1	.123	15	0	1	0	15	0	1
325	11	max	526.853	1	0	1	3.707	1	0	1	.003	1	0	1
326		min	-70.738	3	0	1	.123	15	0	1	0	15	0	1
327	12	max	526.918	1	0	1	3.707	1	0	1	.004	1	0	1
328		min	-70.689	3	0	1	.123	15	0	1	0	15	0	1
329	13	max	526.982	1	0	1	3.707	1	0	1	.004	1	0	1
330		min	-70.64	3	0	1	.123	15	0	1	0	15	0	1
331	14	max	527.047	1	0	1	3.707	1	0	1	.004	1	0	1
332		min	-70.592	3	0	1	.123	15	0	1	0	15	0	1
333	15	max	527.112	1	0	1	3.707	1	0	1	.005	1	0	1
334		min	-70.543	3	0	1	.123	15	0	1	0	15	0	1
335	16	max	527.177	1	0	1	3.707	1	0	1	.005	1	0	1
336		min	-70.495	3	0	1	.123	15	0	1	0	15	0	1
337	17	max	527.241	1	0	1	3.707	1	0	1	.005	1	0	1
338		min	-70.446	3	0	1	.123	15	0	1	0	15	0	1
339	18	max	527.306	1	0	1	3.707	1	0	1	.006	1	0	1
340		min	-70.398	3	0	1	.123	15	0	1	0	15	0	1
341	19	max	527.371	1	0	1	3.707	1	0	1	.006	1	0	1
342		min	-70.349	3	0	1	.123	15	0	1	0	15	0	1
343	M1	1	max	130.459	1	345.226	3	-2.44	15	0	.143	1	.014	1
344		min	4.36	15	-374.1	1	-72.262	1	0	3	.005	15	-.011	3
345	2	max	130.555	1	345.029	3	-2.44	15	0	1	.127	1	.095	1
346		min	4.389	15	-374.363	1	-72.262	1	0	3	.004	15	-.086	3
347	3	max	110.168	1	7.025	9	-2.417	15	0	12	.11	1	.175	1
348		min	3.982	15	-21.474	3	-71.985	1	0	1	.004	15	-.159	3
349	4	max	110.263	1	6.806	9	-2.417	15	0	12	.094	1	.175	1
350		min	4.01	15	-21.67	3	-71.985	1	0	1	.003	15	-.154	3
351	5	max	110.359	1	6.587	9	-2.417	15	0	12	.079	1	.175	1
352		min	4.039	15	-21.867	3	-71.985	1	0	1	.003	15	-.15	3
353	6	max	110.454	1	6.369	9	-2.417	15	0	12	.063	1	.175	1
354		min	4.068	15	-22.064	3	-71.985	1	0	1	.002	15	-.145	3
355	7	max	110.55	1	6.15	9	-2.417	15	0	12	.048	1	.176	1
356		min	4.097	15	-22.261	3	-71.985	1	0	1	.002	15	-.14	3
357	8	max	110.645	1	5.931	9	-2.417	15	0	12	.032	1	.176	1
358		min	4.126	15	-22.458	3	-71.985	1	0	1	.001	15	-.135	3
359	9	max	110.741	1	5.713	9	-2.417	15	0	12	.016	1	.177	1
360		min	4.155	15	-22.654	3	-71.985	1	0	1	0	15	-.13	3
361	10	max	110.836	1	5.494	9	-2.417	15	0	12	0	1	.177	1
362		min	4.183	15	-22.851	3	-71.985	1	0	1	0	10	-.125	3
363	11	max	110.932	1	5.275	9	-2.417	15	0	12	0	12	.178	1
364		min	4.212	15	-23.048	3	-71.985	1	0	1	-.015	1	-.12	3
365	12	max	111.027	1	5.057	9	-2.417	15	0	12	0	12	.178	1
366		min	4.241	15	-23.245	3	-71.985	1	0	1	-.03	1	-.115	3
367	13	max	111.123	1	4.838	9	-2.417	15	0	12	-.001	12	.179	1
368		min	4.27	15	-23.442	3	-71.985	1	0	1	-.046	1	-.11	3
369	14	max	111.218	1	4.619	9	-2.417	15	0	12	-.002	15	.18	1
370		min	4.299	15	-23.638	3	-71.985	1	0	1	-.062	1	-.105	3
371	15	max	111.314	1	4.401	9	-2.417	15	0	12	-.003	15	.183	2
372		min	4.327	15	-23.835	3	-71.985	1	0	1	-.077	1	-.1	3
373	16	max	82.653	2	29.032	10	-2.44	15	0	1	-.003	15	.187	2
374		min	-31.322	3	-88.051	3	-72.586	1	0	12	-.094	1	-.094	3
375	17	max	82.748	2	28.813	10	-2.44	15	0	1	-.004	15	.197	1
376		min	-31.25	3	-88.248	3	-72.586	1	0	12	-.109	1	-.075	3
377	18	max	-4.369	15	423.966	1	-2.498	15	0	3	-.004	15	.107	1
378		min	-130.051	1	-155.852	3	-74.334	1	0	1	-.126	1	-.042	3
379	19	max	-4.34	15	423.704	1	-2.498	15	0	3	-.005	15	.015	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
380		min	-129.956	1	-156.048	3	-74.334	1	0	1	-.142	1	-.008	3
381	M5	max	287.955	1	1139.088	3	-.067	10	0	1	.004	1	.022	3
382		min	8.311	12	-1234.801	1	-29.812	3	0	3	0	10	-.028	1
383		max	288.051	1	1138.891	3	-.067	10	0	1	0	2	.24	1
384		min	8.359	12	-1235.064	1	-29.812	3	0	3	-.003	3	-.225	3
385		max	220.713	1	8.766	9	3.391	3	0	3	0	2	.503	1
386		min	5.818	10	-70.687	3	-.397	2	0	1	-.009	3	-.467	3
387		max	220.809	1	8.547	9	3.391	3	0	3	0	2	.507	1
388		min	5.898	10	-70.883	3	-.397	2	0	1	-.008	3	-.452	3
389		max	220.904	1	8.328	9	3.391	3	0	3	0	2	.512	1
390		min	5.977	10	-71.08	3	-.397	2	0	1	-.008	3	-.437	3
391		max	221	1	8.11	9	3.391	3	0	3	0	2	.517	1
392		min	6.057	10	-71.277	3	-.397	2	0	1	-.007	3	-.421	3
393		max	221.095	1	7.891	9	3.391	3	0	3	0	2	.521	1
394		min	6.136	10	-71.474	3	-.397	2	0	1	-.006	3	-.406	3
395		max	221.191	1	7.672	9	3.391	3	0	3	0	2	.526	1
396		min	6.216	10	-71.671	3	-.397	2	0	1	-.005	3	-.39	3
397		max	221.286	1	7.454	9	3.391	3	0	3	0	2	.531	1
398		min	6.296	10	-71.867	3	-.397	2	0	1	-.005	3	-.375	3
399		max	221.382	1	7.235	9	3.391	3	0	3	0	10	.536	1
400		min	6.375	10	-72.064	3	-.397	2	0	1	-.004	3	-.359	3
401		max	221.477	1	7.016	9	3.391	3	0	3	0	10	.54	1
402		min	6.455	10	-72.261	3	-.397	2	0	1	-.003	3	-.343	3
403		max	221.573	1	6.798	9	3.391	3	0	3	0	10	.545	1
404		min	6.534	10	-72.458	3	-.397	2	0	1	-.003	3	-.328	3
405		max	221.668	1	6.579	9	3.391	3	0	3	0	10	.55	1
406		min	6.614	10	-72.655	3	-.397	2	0	1	-.002	1	-.312	3
407		max	221.764	1	6.36	9	3.391	3	0	3	0	10	.556	1
408		min	6.694	10	-72.851	3	-.397	2	0	1	-.002	1	-.296	3
409		max	221.859	1	6.142	9	3.391	3	0	3	0	15	.561	1
410		min	6.773	10	-73.048	3	-.397	2	0	1	-.002	1	-.28	3
411		max	294.178	2	175.309	2	3.365	3	0	1	0	3	.566	1
412		min	-102.649	3	-264.805	3	-.407	2	0	15	-.001	1	-.263	3
413		max	294.274	2	175.047	2	3.365	3	0	1	0	3	.569	1
414		min	-102.577	3	-265.002	3	-.407	2	0	15	0	1	-.205	3
415		max	-8.937	12	1393.921	1	3.093	3	0	3	.002	3	.272	1
416		min	-288.735	1	-511.63	3	-.097	2	0	1	0	2	-.095	3
417		max	-8.889	12	1393.659	1	3.093	3	0	3	.002	3	.016	3
418		min	-288.639	1	-511.827	3	-.097	2	0	1	0	2	-.03	1
419	M9	max	129.854	1	345.21	3	94.196	1	0	3	-.005	15	.014	1
420		min	4.338	15	-374.084	1	3.323	15	0	1	-.142	1	-.011	3
421		max	129.949	1	345.013	3	94.196	1	0	3	-.003	12	.095	1
422		min	4.367	15	-374.346	1	3.323	15	0	1	-.122	1	-.086	3
423		max	110.141	1	6.999	9	68.024	1	0	1	.003	3	.175	1
424		min	4.151	15	-21.416	3	1.004	12	0	15	-.1	1	-.159	3
425		max	110.236	1	6.78	9	68.024	1	0	1	.003	3	.175	1
426		min	4.18	15	-21.613	3	1.004	12	0	15	-.085	1	-.154	3
427		max	110.332	1	6.561	9	68.024	1	0	1	.004	3	.175	1
428		min	4.209	15	-21.81	3	1.004	12	0	15	-.07	1	-.15	3
429		max	110.427	1	6.343	9	68.024	1	0	1	.004	3	.175	1
430		min	4.238	15	-22.006	3	1.004	12	0	15	-.056	1	-.145	3
431		max	110.523	1	6.124	9	68.024	1	0	1	.004	3	.176	1
432		min	4.267	15	-22.203	3	1.004	12	0	15	-.041	1	-.14	3
433		max	110.618	1	5.905	9	68.024	1	0	1	.004	3	.176	1
434		min	4.296	15	-22.4	3	1.004	12	0	15	-.026	1	-.135	3
435		max	110.714	1	5.687	9	68.024	1	0	1	.005	3	.177	1
436		min	4.324	15	-22.597	3	1.004	12	0	15	-.011	1	-.13	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

	Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
437		10	max	110.809	1	5.468	9	68.024	1	0	1	.005	3	.177	1
438			min	4.353	15	-22.794	3	1.004	12	0	15	0	2	-.125	3
439		11	max	110.905	1	5.249	9	68.024	1	0	1	.018	1	.178	1
440			min	4.382	15	-22.99	3	1.004	12	0	15	0	15	-.12	3
441		12	max	111	1	5.031	9	68.024	1	0	1	.033	1	.178	1
442			min	4.411	15	-23.187	3	1.004	12	0	15	.001	15	-.115	3
443		13	max	111.096	1	4.812	9	68.024	1	0	1	.048	1	.179	1
444			min	4.44	15	-23.384	3	1.004	12	0	15	.002	15	-.11	3
445		14	max	111.191	1	4.593	9	68.024	1	0	1	.062	1	.18	1
446			min	4.468	15	-23.581	3	1.004	12	0	15	.002	15	-.105	3
447		15	max	111.287	1	4.375	9	68.024	1	0	1	.077	1	.183	2
448			min	4.497	15	-23.778	3	1.004	12	0	15	.003	15	-.1	3
449		16	max	82.905	2	28.679	10	68.76	1	0	15	.093	1	.187	2
450			min	-31.416	3	-88.458	3	1.022	12	0	1	.003	15	-.094	3
451		17	max	83	2	28.461	10	68.76	1	0	15	.108	1	.197	1
452			min	-31.344	3	-88.655	3	1.022	12	0	1	.004	15	-.075	3
453		18	max	-4.36	15	423.967	1	72.436	1	0	1	.124	1	.107	1
454			min	-129.769	1	-155.85	3	1.249	12	0	3	.004	15	-.042	3
455		19	max	-4.331	15	423.704	1	72.436	1	0	1	.14	1	.015	1
456			min	-129.673	1	-156.047	3	1.249	12	0	3	.005	15	-.008	3
457	M13	1	max	94.403	1	373.509	1	-4.338	15	.014	1	.142	1	0	1
458			min	3.323	15	-345.199	3	-129.839	1	-.011	3	.005	15	0	3
459		2	max	94.403	1	263.644	1	-3.323	15	.014	1	.043	1	.253	3
460			min	3.323	15	-243.57	3	-99.38	1	-.011	3	.001	15	-.274	1
461		3	max	94.403	1	153.779	1	-2.308	15	.014	1	.002	3	.419	3
462			min	3.323	15	-141.94	3	-68.92	1	-.011	3	-.029	1	-.454	1
463		4	max	94.403	1	43.914	1	-1.293	15	.014	1	0	12	.498	3
464			min	3.323	15	-40.311	3	-38.461	1	-.011	3	-.075	1	-.539	1
465		5	max	94.403	1	61.318	3	-.278	15	.014	1	-.002	12	.489	3
466			min	3.323	15	-65.951	1	-8.002	1	-.011	3	-.095	1	-.53	1
467		6	max	94.403	1	162.947	3	22.458	1	.014	1	-.002	12	.392	3
468			min	3.323	15	-175.817	1	.252	12	-.011	3	-.089	1	-.426	1
469		7	max	94.403	1	264.577	3	52.917	1	.014	1	-.002	12	.208	3
470			min	3.323	15	-285.682	1	1.242	12	-.011	3	-.057	1	-.227	1
471		8	max	94.403	1	366.206	3	83.377	1	.014	1	.002	1	.066	1
472			min	3.323	15	-395.547	1	2.232	12	-.011	3	0	3	-.063	3
473		9	max	94.403	1	467.835	3	113.836	1	.014	1	.087	1	.454	1
474			min	3.323	15	-505.412	1	3.223	12	-.011	3	.002	12	-.422	3
475		10	max	94.403	1	569.465	3	144.295	1	.012	2	.198	1	.937	1
476			min	3.323	15	-615.277	1	4.213	12	-.014	1	.006	12	-.869	3
477		11	max	72.49	1	505.412	1	-3.105	12	.011	3	.083	1	.454	1
478			min	2.44	15	-467.835	3	-113.227	1	-.014	1	0	3	-.422	3
479		12	max	72.49	1	395.547	1	-2.115	12	.011	3	.001	2	.066	1
480			min	2.44	15	-366.206	3	-82.768	1	-.014	1	-.003	3	-.063	3
481		13	max	72.49	1	285.682	1	-1.124	12	.011	3	-.002	15	.208	3
482			min	2.44	15	-264.577	3	-52.308	1	-.014	1	-.059	1	-.227	1
483		14	max	72.49	1	175.817	1	-.108	3	.011	3	-.003	15	.392	3
484			min	2.44	15	-162.947	3	-21.849	1	-.014	1	-.091	1	-.426	1
485		15	max	72.49	1	65.951	1	8.611	1	.011	3	-.003	15	.489	3
486			min	2.44	15	-61.318	3	.3	15	-.014	1	-.097	1	-.53	1
487		16	max	72.49	1	40.311	3	39.07	1	.011	3	-.002	12	.498	3
488			min	2.44	15	-43.914	1	1.315	15	-.014	1	-.076	1	-.539	1
489		17	max	72.49	1	141.94	3	69.529	1	.011	3	0	12	.419	3
490			min	2.44	15	-153.779	1	2.33	15	-.014	1	-.03	1	-.454	1
491		18	max	72.49	1	243.57	3	99.989	1	.011	3	.043	1	.253	3
492			min	2.44	15	-263.644	1	3.345	15	-.014	1	.001	15	-.274	1
493		19	max	72.49	1	345.199	3	130.448	1	.011	3	.143	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
494	M16	min	2.44	15	-373.509	1	4.36	15	-.014	1	.005	15	0	3
495		max	-1.248	12	424.316	1	-4.331	15	.008	3	.14	1	0	1
496		min	-72.189	1	-156.067	3	-129.685	1	-.015	1	.005	15	0	3
497		2 max	-1.248	12	299.492	1	-3.316	15	.008	3	.041	1	.115	3
498		min	-72.189	1	-110.285	3	-99.226	1	-.015	1	.001	15	-.312	1
499		3 max	-1.248	12	174.668	1	-2.301	15	.008	3	0	12	.19	3
500		min	-72.189	1	-64.503	3	-68.766	1	-.015	1	-.031	1	-.516	1
501		4 max	-1.248	12	49.844	1	-1.286	15	.008	3	-.003	15	.226	3
502		min	-72.189	1	-18.721	3	-38.307	1	-.015	1	-.077	1	-.612	1
503		5 max	-1.248	12	27.061	3	-.271	15	.008	3	-.003	15	.222	3
504		min	-72.189	1	-74.98	1	-7.847	1	-.015	1	-.097	1	-.602	1
505		6 max	-1.248	12	72.843	3	22.612	1	.008	3	-.003	15	.179	3
506		min	-72.189	1	-199.804	1	.411	12	-.015	1	-.091	1	-.483	1
507		7 max	-1.248	12	118.625	3	53.071	1	.008	3	-.002	15	.097	3
508		min	-72.189	1	-324.628	1	1.401	12	-.015	1	-.058	1	-.258	1
509		8 max	-1.248	12	164.407	3	83.531	1	.008	3	.002	2	.076	1
510		min	-72.189	1	-449.452	1	2.391	12	-.015	1	-.002	3	-.025	3
511		9 max	-1.248	12	210.189	3	113.99	1	.008	3	.086	1	.517	1
512		min	-72.189	1	-574.276	1	3.382	12	-.015	1	.001	12	-.186	3
513		10 max	-2.498	15	-15.62	15	144.45	1	0	15	.197	1	1.065	1
514		min	-74.115	1	-699.1	1	-6.749	3	-.015	1	.006	12	-.387	3
515		11 max	-2.498	15	574.276	1	-3.526	12	.015	1	.086	1	.517	1
516		min	-74.115	1	-210.189	3	-113.707	1	-.008	3	.002	12	-.186	3
517		12 max	-2.498	15	449.452	1	-2.536	12	.015	1	.001	2	.076	1
518		min	-74.115	1	-164.407	3	-83.248	1	-.008	3	0	3	-.025	3
519		13 max	-2.498	15	324.628	1	-1.546	12	.015	1	-.002	12	.097	3
520		min	-74.115	1	-118.625	3	-52.788	1	-.008	3	-.058	1	-.258	1
521		14 max	-2.498	15	199.804	1	-.555	12	.015	1	-.003	12	.179	3
522		min	-74.115	1	-72.843	3	-22.329	1	-.008	3	-.09	1	-.483	1
523		15 max	-2.498	15	74.98	1	8.13	1	.015	1	-.003	12	.222	3
524		min	-74.115	1	-27.061	3	.28	15	-.008	3	-.096	1	-.602	1
525		16 max	-2.498	15	18.721	3	38.59	1	.015	1	-.002	12	.226	3
526		min	-74.115	1	-49.844	1	1.295	15	-.008	3	-.076	1	-.612	1
527		17 max	-2.498	15	64.503	3	69.049	1	.015	1	0	12	.19	3
528		min	-74.115	1	-174.668	1	2.31	15	-.008	3	-.03	1	-.516	1
529		18 max	-2.498	15	110.285	3	99.509	1	.015	1	.043	1	.115	3
530		min	-74.115	1	-299.492	1	3.325	15	-.008	3	.001	15	-.312	1
531		19 max	-2.498	15	156.067	3	129.968	1	.015	1	.142	1	0	1
532		min	-74.115	1	-424.316	1	4.34	15	-.008	3	.005	15	0	3
533	M15	1 max	0	2	2.7	4	.029	3	0	1	0	1	0	1
534		min	-34.946	3	0	2	-.034	1	0	3	0	3	0	1
535		2 max	0	2	2.4	4	.029	3	0	1	0	1	0	2
536		min	-35.006	3	0	2	-.034	1	0	3	0	3	-.001	4
537		3 max	0	2	2.1	4	.029	3	0	1	0	1	0	2
538		min	-35.066	3	0	2	-.034	1	0	3	0	3	-.002	4
539		4 max	0	2	1.8	4	.029	3	0	1	0	1	0	2
540		min	-35.125	3	0	2	-.034	1	0	3	0	3	-.003	4
541		5 max	0	2	1.5	4	.029	3	0	1	0	1	0	2
542		min	-35.185	3	0	2	-.034	1	0	3	0	3	-.004	4
543		6 max	0	2	1.2	4	.029	3	0	1	0	1	0	2
544		min	-35.245	3	0	2	-.034	1	0	3	0	3	-.004	4
545		7 max	0	2	.9	4	.029	3	0	1	0	3	0	2
546		min	-35.304	3	0	2	-.034	1	0	3	0	1	-.005	4
547		8 max	0	2	.6	4	.029	3	0	1	0	3	0	2
548		min	-35.364	3	0	2	-.034	1	0	3	0	1	-.005	4
549		9 max	0	2	.3	4	.029	3	0	1	0	3	0	2
550		min	-35.424	3	0	2	-.034	1	0	3	0	1	-.005	4



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
551	10	max	0	2	0	1	.029	3	0	1	0	3	0	2
552		min	-35.483	3	0	1	-.034	1	0	3	0	1	-.006	4
553	11	max	0	2	0	2	.029	3	0	1	0	3	0	2
554		min	-35.543	3	-.3	4	-.034	1	0	3	0	1	-.005	4
555	12	max	0	2	0	2	.029	3	0	1	0	3	0	2
556		min	-35.603	3	-.6	4	-.034	1	0	3	0	1	-.005	4
557	13	max	0	2	0	2	.029	3	0	1	0	3	0	2
558		min	-35.662	3	-.9	4	-.034	1	0	3	0	1	-.005	4
559	14	max	0	2	0	2	.029	3	0	1	0	3	0	2
560		min	-35.722	3	-1.2	4	-.034	1	0	3	0	1	-.004	4
561	15	max	0	2	0	2	.029	3	0	1	0	3	0	2
562		min	-35.782	3	-1.5	4	-.034	1	0	3	0	1	-.004	4
563	16	max	0	2	0	2	.029	3	0	1	0	3	0	2
564		min	-35.841	3	-1.8	4	-.034	1	0	3	0	1	-.003	4
565	17	max	0	2	0	2	.029	3	0	1	0	3	0	2
566		min	-35.901	3	-2.1	4	-.034	1	0	3	0	1	-.002	4
567	18	max	0	2	0	2	.029	3	0	1	0	3	0	2
568		min	-35.961	3	-2.4	4	-.034	1	0	3	0	1	-.001	4
569	19	max	0	2	0	2	.029	3	0	1	0	3	0	1
570		min	-36.02	3	-2.7	4	-.034	1	0	3	0	1	0	1
571	M16A	1	max	-7.97	10	2.7	.022	1	0	3	0	3	0	1
572		min	-35.514	3	.635	15	-.012	3	0	1	0	1	0	1
573	2	max	-.731	10	2.4	4	.022	1	0	3	0	3	0	15
574		min	-35.454	3	.564	15	-.012	3	0	1	0	1	-.001	4
575	3	max	-.665	10	2.1	4	.022	1	0	3	0	3	0	15
576		min	-35.395	3	.494	15	-.012	3	0	1	0	1	-.002	4
577	4	max	-.598	10	1.8	4	.022	1	0	3	0	3	0	15
578		min	-35.335	3	.423	15	-.012	3	0	1	0	1	-.003	4
579	5	max	-.532	10	1.5	4	.022	1	0	3	0	3	0	15
580		min	-35.275	3	.353	15	-.012	3	0	1	0	1	-.004	4
581	6	max	-.466	10	1.2	4	.022	1	0	3	0	3	-.001	15
582		min	-35.216	3	.282	15	-.012	3	0	1	0	1	-.004	4
583	7	max	-.4	10	.9	4	.022	1	0	3	0	3	-.001	15
584		min	-35.156	3	.212	15	-.012	3	0	1	0	1	-.005	4
585	8	max	-.333	10	.6	4	.022	1	0	3	0	3	-.001	15
586		min	-35.096	3	.141	15	-.012	3	0	1	0	1	-.005	4
587	9	max	-.267	10	.3	4	.022	1	0	3	0	3	-.001	15
588		min	-35.037	3	.071	15	-.012	3	0	1	0	1	-.005	4
589	10	max	-.201	10	0	1	.022	1	0	3	0	3	-.001	15
590		min	-34.977	3	0	1	-.012	3	0	1	0	1	-.006	4
591	11	max	-.134	10	-.071	15	.022	1	0	3	0	3	-.001	15
592		min	-34.917	3	-.3	4	-.012	3	0	1	0	1	-.005	4
593	12	max	-.068	10	-.141	15	.022	1	0	3	0	3	-.001	15
594		min	-34.858	3	-.6	4	-.012	3	0	1	0	1	-.005	4
595	13	max	-.002	10	-.212	15	.022	1	0	3	0	2	-.001	15
596		min	-34.798	3	-.9	4	-.012	3	0	1	0	4	-.005	4
597	14	max	.065	10	-.282	15	.022	1	0	3	0	1	-.001	15
598		min	-34.738	3	-1.2	4	-.012	3	0	1	0	3	-.004	4
599	15	max	.131	10	-.353	15	.022	1	0	3	0	1	0	15
600		min	-34.679	3	-1.5	4	-.012	3	0	1	0	3	-.004	4
601	16	max	.197	10	-.423	15	.022	1	0	3	0	1	0	15
602		min	-34.619	3	-1.8	4	-.012	3	0	1	0	3	-.003	4
603	17	max	.263	10	-.494	15	.022	1	0	3	0	1	0	15
604		min	-34.559	3	-2.1	4	-.012	3	0	1	0	3	-.002	4
605	18	max	.33	10	-.564	15	.022	1	0	3	0	1	0	15
606		min	-34.5	3	-2.4	4	-.012	3	0	1	0	3	-.001	4
607	19	max	.396	10	-.635	15	.022	1	0	3	0	1	0	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Forces (Continued)

Member	Sec		Axial[lb]	LC	y Shear[lb]	LC	z Shear[lb]	LC	Torque[k-ft]	LC	y-y Mome...	LC	z-z Mome...	LC
608		min	-34.44	3	-2.7	4	-.012	3	0	1	0	3	0	1

Envelope Member Section Deflections

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
1	M2	1	max	.003	1	.008	2	.014	1	-3.657e-5	15	NC	3	NC	3
2			min	-.003	3	-.007	3	0	3	-1.086e-3	1	4394.829	2	2424.162	1
3		2	max	.003	1	.007	2	.013	1	-3.506e-5	15	NC	3	NC	3
4			min	-.003	3	-.006	3	0	3	-1.042e-3	1	4763.78	2	2623.705	1
5		3	max	.003	1	.006	2	.012	1	-3.356e-5	15	NC	3	NC	3
6			min	-.003	3	-.006	3	0	3	-9.976e-4	1	5197.02	2	2858.688	1
7		4	max	.003	1	.006	2	.011	1	-3.205e-5	15	NC	3	NC	3
8			min	-.002	3	-.006	3	0	3	-9.532e-4	1	5709.119	2	3137.713	1
9		5	max	.002	1	.005	2	.01	1	-3.054e-5	15	NC	3	NC	3
10			min	-.002	3	-.005	3	0	3	-9.088e-4	1	6319.237	2	3472.264	1
11		6	max	.002	1	.005	2	.009	1	-2.904e-5	15	NC	1	NC	3
12			min	-.002	3	-.005	3	0	3	-8.645e-4	1	7052.973	2	3877.947	1
13		7	max	.002	1	.004	2	.008	1	-2.753e-5	15	NC	1	NC	2
14			min	-.002	3	-.005	3	0	3	-8.201e-4	1	7945.163	2	4376.419	1
15		8	max	.002	1	.004	2	.007	1	-2.602e-5	15	NC	1	NC	2
16			min	-.002	3	-.005	3	0	3	-7.757e-4	1	9044.26	2	4998.461	1
17		9	max	.002	1	.003	2	.006	1	-2.451e-5	15	NC	1	NC	2
18			min	-.002	3	-.004	3	0	3	-7.313e-4	1	NC	1	5789.062	1
19		10	max	.002	1	.003	2	.005	1	-2.301e-5	15	NC	1	NC	2
20			min	-.001	3	-.004	3	0	3	-6.87e-4	1	NC	1	6816.191	1
21		11	max	.001	1	.002	2	.004	1	-2.15e-5	15	NC	1	NC	2
22			min	-.001	3	-.004	3	0	3	-6.426e-4	1	NC	1	8186.671	1
23		12	max	.001	1	.002	2	.003	1	-1.999e-5	15	NC	1	NC	1
24			min	-.001	3	-.003	3	0	3	-5.982e-4	1	NC	1	NC	1
25		13	max	.001	1	.002	2	.003	1	-1.849e-5	15	NC	1	NC	1
26			min	0	3	-.003	3	0	3	-5.539e-4	1	NC	1	NC	1
27		14	max	0	1	.001	2	.002	1	-1.698e-5	15	NC	1	NC	1
28			min	0	3	-.002	3	0	3	-5.095e-4	1	NC	1	NC	1
29		15	max	0	1	0	2	.001	1	-1.547e-5	15	NC	1	NC	1
30			min	0	3	-.002	3	0	3	-4.651e-4	1	NC	1	NC	1
31		16	max	0	1	0	2	0	1	-1.396e-5	15	NC	1	NC	1
32			min	0	3	-.002	3	0	3	-4.207e-4	1	NC	1	NC	1
33		17	max	0	1	0	2	0	1	-1.246e-5	15	NC	1	NC	1
34			min	0	3	-.001	3	0	3	-3.764e-4	1	NC	1	NC	1
35		18	max	0	1	0	2	0	1	-1.095e-5	15	NC	1	NC	1
36			min	0	3	0	3	0	12	-3.32e-4	1	NC	1	NC	1
37		19	max	0	1	0	1	0	1	-8.345e-6	12	NC	1	NC	1
38			min	0	1	0	1	0	1	-2.876e-4	1	NC	1	NC	1
39	M3	1	max	0	1	0	1	0	1	1.322e-4	1	NC	1	NC	1
40			min	0	1	0	1	0	1	3.944e-6	12	NC	1	NC	1
41		2	max	0	9	0	2	0	12	1.663e-4	1	NC	1	NC	1
42			min	0	2	0	3	0	1	5.497e-6	15	NC	1	NC	1
43		3	max	0	9	0	2	0	12	2.003e-4	1	NC	1	NC	1
44			min	0	2	-.002	3	0	1	6.653e-6	15	NC	1	NC	1
45		4	max	0	9	0	2	0	12	2.344e-4	1	NC	1	NC	1
46			min	0	2	-.002	3	-.001	1	7.809e-6	15	NC	1	NC	1
47		5	max	0	9	0	2	0	12	2.685e-4	1	NC	1	NC	1
48			min	0	2	-.003	3	-.001	1	8.965e-6	15	NC	1	NC	1
49		6	max	0	9	0	2	0	3	3.026e-4	1	NC	1	NC	1
50			min	0	2	-.004	3	-.001	1	1.012e-5	15	NC	1	NC	1
51		7	max	0	9	.001	2	0	3	3.367e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
52		min	0	2	-.004	3	0	1	1.128e-5	15	NC	1	NC	1
53	8	max	0	9	.001	2	0	3	3.708e-4	1	NC	1	NC	1
54		min	0	2	-.005	3	0	1	1.243e-5	15	NC	1	NC	1
55	9	max	0	9	.002	2	0	3	4.048e-4	1	NC	1	NC	1
56		min	0	2	-.006	3	0	1	1.359e-5	15	NC	1	NC	1
57	10	max	0	9	.002	2	0	1	4.389e-4	1	NC	1	NC	1
58		min	0	2	-.006	3	0	15	1.474e-5	15	NC	1	NC	1
59	11	max	0	9	.003	2	.001	1	4.73e-4	1	NC	1	NC	1
60		min	0	2	-.007	3	0	15	1.59e-5	15	NC	1	NC	1
61	12	max	0	9	.003	2	.002	1	5.071e-4	1	NC	1	NC	1
62		min	0	2	-.007	3	0	15	1.706e-5	15	NC	1	NC	1
63	13	max	0	9	.004	2	.003	1	5.412e-4	1	NC	1	NC	1
64		min	0	2	-.007	3	0	15	1.821e-5	15	NC	1	NC	1
65	14	max	0	9	.005	2	.003	1	5.753e-4	1	NC	1	NC	1
66		min	0	2	-.007	3	0	15	1.937e-5	15	9570.672	2	NC	1
67	15	max	0	9	.006	2	.004	1	6.093e-4	1	NC	3	NC	1
68		min	0	2	-.008	3	0	15	2.052e-5	15	8086.666	2	NC	1
69	16	max	0	9	.007	2	.005	1	6.434e-4	1	NC	3	NC	2
70		min	0	2	-.008	3	0	15	2.168e-5	15	6928.368	2	9500.865	1
71	17	max	0	9	.008	2	.006	1	6.775e-4	1	NC	3	NC	2
72		min	0	2	-.008	3	0	15	2.284e-5	15	6015.812	2	8202.277	1
73	18	max	.001	9	.009	2	.006	1	7.116e-4	1	NC	3	NC	2
74		min	0	2	-.008	3	0	15	2.399e-5	15	5291.002	2	7248.677	1
75	19	max	.001	9	.01	2	.007	1	7.457e-4	1	NC	3	NC	2
76		min	0	2	-.008	3	0	15	2.515e-5	15	4711.647	2	6533.94	1
77	M4	1	max	.003	1	.009	2	15	-3.023e-5	15	NC	1	NC	2
78		min	0	3	-.007	3	-.005	1	-9.085e-4	1	NC	1	3666.994	1
79	2	max	.002	1	.008	2	0	15	-3.023e-5	15	NC	1	NC	2
80		min	0	3	-.006	3	-.005	1	-9.085e-4	1	NC	1	3999.91	1
81	3	max	.002	1	.008	2	0	15	-3.023e-5	15	NC	1	NC	2
82		min	0	3	-.006	3	-.004	1	-9.085e-4	1	NC	1	4396.154	1
83	4	max	.002	1	.007	2	0	15	-3.023e-5	15	NC	1	NC	2
84		min	0	3	-.006	3	-.004	1	-9.085e-4	1	NC	1	4872.426	1
85	5	max	.002	1	.007	2	0	15	-3.023e-5	15	NC	1	NC	2
86		min	0	3	-.005	3	-.004	1	-9.085e-4	1	NC	1	5451.477	1
87	6	max	.002	1	.006	2	0	15	-3.023e-5	15	NC	1	NC	2
88		min	0	3	-.005	3	-.003	1	-9.085e-4	1	NC	1	6164.949	1
89	7	max	.002	1	.006	2	0	15	-3.023e-5	15	NC	1	NC	2
90		min	0	3	-.005	3	-.003	1	-9.085e-4	1	NC	1	7057.917	1
91	8	max	.002	1	.005	2	0	15	-3.023e-5	15	NC	1	NC	2
92		min	0	3	-.004	3	-.002	1	-9.085e-4	1	NC	1	8196.385	1
93	9	max	.001	1	.005	2	0	15	-3.023e-5	15	NC	1	NC	2
94		min	0	3	-.004	3	-.002	1	-9.085e-4	1	NC	1	9680.203	1
95	10	max	.001	1	.004	2	0	15	-3.023e-5	15	NC	1	NC	1
96		min	0	3	-.003	3	-.002	1	-9.085e-4	1	NC	1	NC	1
97	11	max	.001	1	.004	2	0	15	-3.023e-5	15	NC	1	NC	1
98		min	0	3	-.003	3	-.001	1	-9.085e-4	1	NC	1	NC	1
99	12	max	0	1	.003	2	0	15	-3.023e-5	15	NC	1	NC	1
100		min	0	3	-.003	3	-.001	1	-9.085e-4	1	NC	1	NC	1
101	13	max	0	1	.003	2	0	15	-3.023e-5	15	NC	1	NC	1
102		min	0	3	-.002	3	0	1	-9.085e-4	1	NC	1	NC	1
103	14	max	0	1	.002	2	0	15	-3.023e-5	15	NC	1	NC	1
104		min	0	3	-.002	3	0	1	-9.085e-4	1	NC	1	NC	1
105	15	max	0	1	.002	2	0	15	-3.023e-5	15	NC	1	NC	1
106		min	0	3	-.002	3	0	1	-9.085e-4	1	NC	1	NC	1
107	16	max	0	1	.001	2	0	15	-3.023e-5	15	NC	1	NC	1
108		min	0	3	-.001	3	0	1	-9.085e-4	1	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
109		17	max	0	1	0	2	0	15	-3.023e-5	15	NC	1	NC	1
110			min	0	3	0	3	0	1	-9.085e-4	1	NC	1	NC	1
111		18	max	0	1	0	2	0	15	-3.023e-5	15	NC	1	NC	1
112			min	0	3	0	3	0	1	-9.085e-4	1	NC	1	NC	1
113		19	max	0	1	0	1	0	1	-3.023e-5	15	NC	1	NC	1
114			min	0	1	0	1	0	1	-9.085e-4	1	NC	1	NC	1
115	M6	1	max	.01	1	.024	2	.004	1	1.987e-4	3	NC	3	NC	2
116			min	-.01	3	-.019	3	-.003	3	2.554e-6	10	1372.28	2	7794.929	1
117		2	max	.009	1	.023	2	.004	1	1.933e-4	3	NC	3	NC	2
118			min	-.009	3	-.018	3	-.002	3	1.846e-6	10	1465.013	2	8452.291	1
119		3	max	.009	1	.021	2	.004	1	1.879e-4	3	NC	3	NC	2
120			min	-.009	3	-.017	3	-.002	3	1.137e-6	10	1570.853	2	9231.51	1
121		4	max	.008	1	.02	2	.003	1	1.825e-4	3	NC	3	NC	1
122			min	-.008	3	-.016	3	-.002	3	4.291e-7	10	1692.419	2	NC	1
123		5	max	.008	1	.018	2	.003	1	1.771e-4	3	NC	3	NC	1
124			min	-.008	3	-.015	3	-.002	3	-2.792e-7	10	1833.084	2	NC	1
125		6	max	.007	1	.017	2	.003	1	1.717e-4	3	NC	3	NC	1
126			min	-.007	3	-.014	3	-.002	3	-9.874e-7	10	1997.264	2	NC	1
127		7	max	.007	1	.015	2	.002	1	1.664e-4	3	NC	3	NC	1
128			min	-.007	3	-.013	3	-.002	3	-3.539e-6	2	2190.85	2	NC	1
129		8	max	.006	1	.014	2	.002	1	1.61e-4	3	NC	3	NC	1
130			min	-.006	3	-.012	3	-.001	3	-7.177e-6	2	2421.88	2	NC	1
131		9	max	.006	1	.012	2	.002	1	1.556e-4	3	NC	3	NC	1
132			min	-.005	3	-.011	3	-.001	3	-1.082e-5	2	2701.608	2	NC	1
133		10	max	.005	1	.011	2	.001	1	1.502e-4	3	NC	3	NC	1
134			min	-.005	3	-.01	3	-.001	3	-1.445e-5	2	3046.296	2	NC	1
135		11	max	.004	1	.01	2	.001	1	1.448e-4	3	NC	3	NC	1
136			min	-.004	3	-.009	3	0	3	-1.809e-5	2	3480.333	2	NC	1
137		12	max	.004	1	.008	2	0	1	1.394e-4	3	NC	3	NC	1
138			min	-.004	3	-.008	3	0	3	-2.173e-5	2	4042.049	2	NC	1
139		13	max	.003	1	.007	2	0	1	1.34e-4	3	NC	3	NC	1
140			min	-.003	3	-.007	3	0	3	-2.537e-5	2	4795.323	2	NC	1
141		14	max	.003	1	.006	2	0	1	1.286e-4	3	NC	3	NC	1
142			min	-.003	3	-.006	3	0	3	-2.901e-5	2	5855.131	2	NC	1
143		15	max	.002	1	.004	2	0	1	1.232e-4	3	NC	3	NC	1
144			min	-.002	3	-.005	3	0	3	-3.265e-5	2	7451.412	2	NC	1
145		16	max	.002	1	.003	2	0	1	1.179e-4	3	NC	1	NC	1
146			min	-.002	3	-.003	3	0	3	-3.629e-5	2	NC	1	NC	1
147		17	max	.001	1	.002	2	0	1	1.125e-4	3	NC	1	NC	1
148			min	-.001	3	-.002	3	0	3	-3.993e-5	2	NC	1	NC	1
149		18	max	0	1	.001	2	0	1	1.071e-4	3	NC	1	NC	1
150			min	0	3	-.001	3	0	3	-4.356e-5	2	NC	1	NC	1
151		19	max	0	1	0	1	0	1	1.017e-4	3	NC	1	NC	1
152			min	0	1	0	1	0	1	-4.72e-5	2	NC	1	NC	1
153	M7	1	max	0	1	0	1	0	1	2.146e-5	2	NC	1	NC	1
154			min	0	1	0	1	0	1	-4.653e-5	3	NC	1	NC	1
155		2	max	0	3	.001	2	0	3	1.933e-5	1	NC	1	NC	1
156			min	0	2	-.002	3	0	1	-3.506e-5	3	NC	1	NC	1
157		3	max	0	3	.003	2	0	3	1.836e-5	1	NC	1	NC	1
158			min	0	2	-.003	3	0	1	-2.36e-5	3	NC	1	NC	1
159		4	max	0	3	.004	2	0	3	1.738e-5	1	NC	1	NC	1
160			min	0	2	-.005	3	0	1	-1.214e-5	3	NC	1	NC	1
161		5	max	0	3	.005	2	0	3	1.64e-5	1	NC	3	NC	1
162			min	0	2	-.006	3	0	1	-6.708e-7	3	8607.816	2	NC	1
163		6	max	0	3	.007	2	0	3	1.542e-5	1	NC	3	NC	1
164			min	-.001	2	-.008	3	0	1	4.473e-7	15	6898.406	2	NC	1
165		7	max	.001	3	.008	2	.001	3	2.226e-5	3	NC	3	NC	1





Company : Schletter, Inc.
 Designer : HCV
 Job Number :
 Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
223		17	max	0	1	.003	2	0	1	-1.616e-6	10	NC	1	NC	1
224			min	0	3	-.002	3	0	3	-1.256e-4	3	NC	1	NC	1
225		18	max	0	1	.002	2	0	1	-1.616e-6	10	NC	1	NC	1
226			min	0	3	-.001	3	0	3	-1.256e-4	3	NC	1	NC	1
227		19	max	0	1	0	1	0	1	-1.616e-6	10	NC	1	NC	1
228			min	0	1	0	1	0	1	-1.256e-4	3	NC	1	NC	1
229	M10	1	max	.003	1	.008	2	0	3	9.488e-4	1	NC	3	NC	1
230			min	-.003	3	-.007	3	-.002	1	-2.053e-4	3	4400.19	2	NC	1
231		2	max	.003	1	.007	2	0	3	8.999e-4	1	NC	3	NC	1
232			min	-.003	3	-.006	3	-.002	1	-1.995e-4	3	4769.706	2	NC	1
233		3	max	.003	1	.006	2	0	3	8.509e-4	1	NC	3	NC	1
234			min	-.003	3	-.006	3	-.002	1	-1.937e-4	3	5203.63	2	NC	1
235		4	max	.003	1	.006	2	0	3	8.02e-4	1	NC	3	NC	1
236			min	-.002	3	-.006	3	-.002	1	-1.879e-4	3	5716.562	2	NC	1
237		5	max	.002	1	.005	2	0	3	7.53e-4	1	NC	3	NC	1
238			min	-.002	3	-.006	3	-.001	1	-1.821e-4	3	6327.705	2	NC	1
239		6	max	.002	1	.005	2	0	3	7.041e-4	1	NC	1	NC	1
240			min	-.002	3	-.005	3	-.001	1	-1.763e-4	3	7062.713	2	NC	1
241		7	max	.002	1	.004	2	0	3	6.551e-4	1	NC	1	NC	1
242			min	-.002	3	-.005	3	-.001	1	-1.705e-4	3	7956.505	2	NC	1
243		8	max	.002	1	.004	2	0	3	6.062e-4	1	NC	1	NC	1
244			min	-.002	3	-.005	3	-.001	1	-1.647e-4	3	9057.645	2	NC	1
245		9	max	.002	1	.003	2	0	3	5.572e-4	1	NC	1	NC	1
246			min	-.002	3	-.004	3	0	1	-1.59e-4	3	NC	1	NC	1
247		10	max	.002	1	.003	2	0	3	5.083e-4	1	NC	1	NC	1
248			min	-.001	3	-.004	3	0	1	-1.532e-4	3	NC	1	NC	1
249		11	max	.001	1	.002	2	0	3	4.593e-4	1	NC	1	NC	1
250			min	-.001	3	-.004	3	0	1	-1.474e-4	3	NC	1	NC	1
251		12	max	.001	1	.002	2	0	3	4.104e-4	1	NC	1	NC	1
252			min	-.001	3	-.003	3	0	1	-1.416e-4	3	NC	1	NC	1
253		13	max	.001	1	.002	2	0	3	3.614e-4	1	NC	1	NC	1
254			min	0	3	-.003	3	0	1	-1.358e-4	3	NC	1	NC	1
255		14	max	0	1	.001	2	0	3	3.125e-4	1	NC	1	NC	1
256			min	0	3	-.002	3	0	1	-1.3e-4	3	NC	1	NC	1
257		15	max	0	1	0	2	0	3	2.635e-4	1	NC	1	NC	1
258			min	0	3	-.002	3	0	1	-1.242e-4	3	NC	1	NC	1
259		16	max	0	1	0	2	0	3	2.146e-4	1	NC	1	NC	1
260			min	0	3	-.002	3	0	1	-1.184e-4	3	NC	1	NC	1
261		17	max	0	1	0	2	0	3	1.656e-4	1	NC	1	NC	1
262			min	0	3	-.001	3	0	1	-1.126e-4	3	NC	1	NC	1
263		18	max	0	1	0	2	0	3	1.167e-4	1	NC	1	NC	1
264			min	0	3	0	3	0	1	-1.069e-4	3	NC	1	NC	1
265		19	max	0	1	0	1	0	1	6.771e-5	1	NC	1	NC	1
266			min	0	1	0	1	0	1	-1.011e-4	3	NC	1	NC	1
267	M11	1	max	0	1	0	1	0	1	4.65e-5	3	NC	1	NC	1
268			min	0	1	0	1	0	1	-3.248e-5	1	NC	1	NC	1
269		2	max	0	9	0	2	0	1	3.327e-5	3	NC	1	NC	1
270			min	0	2	0	3	0	3	-9.313e-5	1	NC	1	NC	1
271		3	max	0	9	0	2	0	10	2.003e-5	3	NC	1	NC	1
272			min	0	2	-.002	3	0	3	-1.538e-4	1	NC	1	NC	1
273		4	max	0	9	0	2	0	10	6.8e-6	3	NC	1	NC	1
274			min	0	2	-.002	3	0	3	-2.144e-4	1	NC	1	NC	1
275		5	max	0	9	0	2	0	10	-4.778e-6	12	NC	1	NC	1
276			min	0	2	-.003	3	0	1	-2.751e-4	1	NC	1	NC	1
277		6	max	0	9	0	2	0	15	-1.121e-5	15	NC	1	NC	1
278			min	0	2	-.004	3	-.001	1	-3.357e-4	1	NC	1	NC	1
279		7	max	0	9	.001	2	0	15	-1.329e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
280			min	0	2	-.005	3	-.002	1	-3.964e-4	1	NC	1	NC	1
281		8	max	0	9	.001	2	0	15	-1.538e-5	15	NC	1	NC	1
282			min	0	2	-.005	3	-.003	1	-4.571e-4	1	NC	1	NC	1
283		9	max	0	9	.002	2	0	15	-1.747e-5	15	NC	1	NC	1
284			min	0	2	-.006	3	-.004	1	-5.177e-4	1	NC	1	NC	1
285		10	max	0	9	.002	2	0	15	-1.955e-5	15	NC	1	NC	2
286			min	0	2	-.006	3	-.005	1	-5.784e-4	1	NC	1	9272.676	1
287		11	max	0	9	.003	2	0	15	-2.164e-5	15	NC	1	NC	2
288			min	0	2	-.007	3	-.006	1	-6.39e-4	1	NC	1	7655.575	1
289		12	max	0	9	.003	2	0	15	-2.372e-5	15	NC	1	NC	2
290			min	0	2	-.007	3	-.007	1	-6.997e-4	1	NC	1	6486.111	1
291		13	max	0	9	.004	2	0	15	-2.581e-5	15	NC	1	NC	2
292			min	0	2	-.007	3	-.008	1	-7.603e-4	1	NC	1	5614.168	1
293		14	max	0	9	.005	2	0	15	-2.79e-5	15	NC	1	NC	2
294			min	0	2	-.007	3	-.009	1	-8.21e-4	1	9583.857	2	4948.514	1
295		15	max	0	9	.006	2	0	15	-2.998e-5	15	NC	3	NC	2
296			min	0	2	-.008	3	-.01	1	-8.816e-4	1	8096.695	2	4431.105	1
297		16	max	0	9	.007	2	0	15	-3.207e-5	15	NC	3	NC	2
298			min	0	2	-.008	3	-.011	1	-9.423e-4	1	6936.177	2	4023.629	1
299		17	max	0	9	.008	2	0	15	-3.415e-5	15	NC	3	NC	2
300			min	0	2	-.008	3	-.012	1	-1.003e-3	1	6022.033	2	3700.061	1
301		18	max	.001	9	.009	2	0	15	-3.624e-5	15	NC	3	NC	3
302			min	0	2	-.008	3	-.013	1	-1.064e-3	1	5296.071	2	3442.33	1
303		19	max	.001	9	.01	2	0	15	-3.833e-5	15	NC	3	NC	3
304			min	0	2	-.008	3	-.014	1	-1.124e-3	1	4715.871	2	3237.728	1
305	M12	1	max	.003	1	.009	2	.012	1	9.739e-4	1	NC	1	NC	3
306			min	0	3	-.007	3	0	15	3.372e-5	15	NC	1	1639.004	1
307		2	max	.002	1	.008	2	.011	1	9.739e-4	1	NC	1	NC	3
308			min	0	3	-.006	3	0	15	3.372e-5	15	NC	1	1787.353	1
309		3	max	.002	1	.008	2	.01	1	9.739e-4	1	NC	1	NC	3
310			min	0	3	-.006	3	0	15	3.372e-5	15	NC	1	1963.943	1
311		4	max	.002	1	.007	2	.009	1	9.739e-4	1	NC	1	NC	3
312			min	0	3	-.006	3	0	15	3.372e-5	15	NC	1	2176.217	1
313		5	max	.002	1	.007	2	.008	1	9.739e-4	1	NC	1	NC	3
314			min	0	3	-.005	3	0	15	3.372e-5	15	NC	1	2434.314	1
315		6	max	.002	1	.006	2	.007	1	9.739e-4	1	NC	1	NC	3
316			min	0	3	-.005	3	0	15	3.372e-5	15	NC	1	2752.339	1
317		7	max	.002	1	.006	2	.006	1	9.739e-4	1	NC	1	NC	3
318			min	0	3	-.005	3	0	15	3.372e-5	15	NC	1	3150.38	1
319		8	max	.002	1	.005	2	.005	1	9.739e-4	1	NC	1	NC	3
320			min	0	3	-.004	3	0	15	3.372e-5	15	NC	1	3657.854	1
321		9	max	.001	1	.005	2	.004	1	9.739e-4	1	NC	1	NC	2
322			min	0	3	-.004	3	0	15	3.372e-5	15	NC	1	4319.26	1
323		10	max	.001	1	.004	2	.004	1	9.739e-4	1	NC	1	NC	2
324			min	0	3	-.003	3	0	15	3.372e-5	15	NC	1	5204.558	1
325		11	max	.001	1	.004	2	.003	1	9.739e-4	1	NC	1	NC	2
326			min	0	3	-.003	3	0	15	3.372e-5	15	NC	1	6428.981	1
327		12	max	0	1	.003	2	.002	1	9.739e-4	1	NC	1	NC	2
328			min	0	3	-.003	3	0	15	3.372e-5	15	NC	1	8193.553	1
329		13	max	0	1	.003	2	.002	1	9.739e-4	1	NC	1	NC	1
330			min	0	3	-.002	3	0	15	3.372e-5	15	NC	1	NC	1
331		14	max	0	1	.002	2	.001	1	9.739e-4	1	NC	1	NC	1
332			min	0	3	-.002	3	0	15	3.372e-5	15	NC	1	NC	1
333		15	max	0	1	.002	2	0	1	9.739e-4	1	NC	1	NC	1
334			min	0	3	-.002	3	0	15	3.372e-5	15	NC	1	NC	1
335		16	max	0	1	.001	2	0	1	9.739e-4	1	NC	1	NC	1
336			min	0	3	-.001	3	0	15	3.372e-5	15	NC	1	NC	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
337		17	max	0	1	0	2	0	1	9.739e-4	1	NC	1	NC	1
338			min	0	3	0	3	0	15	3.372e-5	15	NC	1	NC	1
339		18	max	0	1	0	2	0	1	9.739e-4	1	NC	1	NC	1
340			min	0	3	0	3	0	15	3.372e-5	15	NC	1	NC	1
341		19	max	0	1	0	1	0	1	9.739e-4	1	NC	1	NC	1
342			min	0	1	0	1	0	1	3.372e-5	15	NC	1	NC	1
343	M1	1	max	.006	3	.023	3	.001	3	1.567e-2	1	NC	1	NC	1
344			min	-.007	2	-.028	1	-.005	1	-1.438e-2	3	NC	1	NC	1
345		2	max	.006	3	.013	3	0	3	7.446e-3	1	NC	4	NC	2
346			min	-.007	2	-.015	1	-.01	1	-7.115e-3	3	3541.86	1	8396.051	1
347		3	max	.006	3	.003	3	0	3	1.565e-5	3	NC	4	NC	2
348			min	-.007	2	-.003	1	-.014	1	-6.212e-4	1	1831.876	1	5090.975	1
349		4	max	.006	3	.007	1	0	3	1.884e-5	3	NC	5	NC	2
350			min	-.007	2	-.005	3	-.016	1	-5.189e-4	1	1295.784	1	4212.122	1
351		5	max	.006	3	.016	1	0	3	2.202e-5	3	NC	5	NC	2
352			min	-.007	2	-.012	3	-.016	1	-4.166e-4	1	1038.123	1	4043.72	1
353		6	max	.006	3	.023	1	0	3	2.52e-5	3	NC	5	NC	2
354			min	-.007	2	-.017	3	-.015	1	-3.143e-4	1	892.513	1	4325.28	1
355		7	max	.006	3	.029	1	0	3	2.839e-5	3	NC	5	NC	2
356			min	-.007	2	-.021	3	-.013	1	-2.12e-4	1	804.383	1	5146.843	1
357		8	max	.006	3	.033	1	0	3	3.157e-5	3	NC	5	NC	2
358			min	-.007	2	-.024	3	-.011	1	-1.097e-4	1	751.042	1	7056.205	1
359		9	max	.006	3	.035	1	0	3	3.476e-5	3	NC	5	NC	1
360			min	-.007	2	-.025	3	-.008	1	-9.31e-6	2	721.897	1	NC	1
361		10	max	.006	3	.036	1	0	3	9.49e-5	1	NC	5	NC	1
362			min	-.007	2	-.025	3	-.005	1	3.477e-6	15	712.063	1	NC	1
363		11	max	.006	3	.035	1	0	3	1.972e-4	1	NC	5	NC	1
364			min	-.008	2	-.024	3	-.001	1	6.894e-6	15	720.059	1	NC	1
365		12	max	.006	3	.033	1	.002	1	2.995e-4	1	NC	5	NC	2
366			min	-.008	2	-.022	3	0	15	1.031e-5	15	747.19	1	8161.411	1
367		13	max	.006	3	.029	1	.004	1	4.018e-4	1	NC	5	NC	2
368			min	-.008	2	-.019	3	0	15	1.373e-5	15	798.107	1	5669.259	1
369		14	max	.006	3	.023	1	.006	1	5.041e-4	1	NC	5	NC	2
370			min	-.008	2	-.015	3	0	15	1.715e-5	15	883.006	1	4649.058	1
371		15	max	.006	3	.015	1	.007	1	6.064e-4	1	NC	5	NC	2
372			min	-.008	2	-.01	3	0	15	2.057e-5	15	1023.756	1	4282.421	1
373		16	max	.006	3	.006	1	.007	1	6.805e-4	1	NC	5	NC	2
374			min	-.008	2	-.004	3	0	15	2.306e-5	15	1272.541	1	4413.787	1
375		17	max	.006	3	.002	3	.005	1	8.322e-5	1	NC	4	NC	2
376			min	-.008	2	-.005	2	0	15	3.499e-6	15	1786.465	1	5295.631	1
377		18	max	.006	3	.01	3	.002	1	8.845e-3	1	NC	4	NC	2
378			min	-.008	2	-.017	1	0	15	-3.293e-3	3	3443.067	1	8689.363	1
379		19	max	.006	3	.018	3	0	3	1.778e-2	1	NC	1	NC	1
380			min	-.008	2	-.031	1	-.003	1	-6.679e-3	3	NC	1	NC	1
381	M5	1	max	.018	3	.069	3	.001	3	6.855e-7	3	NC	1	NC	1
382			min	-.024	2	-.085	1	-.005	1	4.254e-8	15	NC	1	NC	1
383		2	max	.018	3	.039	3	.002	3	5.137e-5	3	NC	5	NC	1
384			min	-.024	2	-.047	1	-.005	1	-8.444e-5	1	1200.261	1	NC	1
385		3	max	.018	3	.01	3	.003	3	1.011e-4	3	NC	5	NC	1
386			min	-.024	2	-.011	1	-.004	1	-1.677e-4	1	617.77	1	NC	1
387		4	max	.018	3	.021	1	.003	3	9.954e-5	3	NC	5	NC	1
388			min	-.024	2	-.013	3	-.004	1	-1.574e-4	1	435.844	1	NC	1
389		5	max	.018	3	.047	1	.003	3	9.799e-5	3	NC	15	NC	1
390			min	-.024	2	-.033	3	-.003	1	-1.472e-4	1	348.364	1	NC	1
391		6	max	.018	3	.069	1	.003	3	9.645e-5	3	NC	15	NC	1
392			min	-.024	2	-.048	3	-.003	1	-1.369e-4	1	298.846	1	NC	1
393		7	max	.018	3	.086	1	.004	3	9.49e-5	3	NC	15	NC	1





Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

	Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
451		17	max	.006	3	.003	3	0	15	-1.087e-5	12	NC	4	NC	2
452			min	-.008	2	-.005	2	-.012	1	-5.293e-4	1	1786.499	1	4990.47	1
453		18	max	.006	3	.01	3	0	15	3.301e-3	3	NC	4	NC	2
454			min	-.008	2	-.017	1	-.008	1	-9.071e-3	1	3443.126	1	8293.421	1
455		19	max	.006	3	.018	3	0	3	6.679e-3	3	NC	1	NC	1
456			min	-.008	2	-.031	1	-.002	1	-1.778e-2	1	NC	1	NC	1
457	M13	1	max	.006	1	.023	3	.006	3	4.003e-3	3	NC	1	NC	1
458			min	0	3	-.029	1	-.007	2	-4.991e-3	1	NC	1	NC	1
459		2	max	.006	1	.167	3	.039	1	4.831e-3	3	NC	5	NC	2
460			min	-.001	3	-.185	1	0	10	-6.053e-3	1	1190.053	1	4274.732	1
461		3	max	.006	1	.284	3	.098	1	5.66e-3	3	NC	5	NC	3
462			min	-.001	3	-.313	1	.003	15	-7.114e-3	1	653.951	1	1802.279	1
463		4	max	.006	1	.358	3	.148	1	6.489e-3	3	NC	5	NC	3
464			min	-.001	3	-.394	1	.005	15	-8.176e-3	1	509.049	1	1213.125	1
465		5	max	.006	1	.38	3	.172	1	7.317e-3	3	NC	5	NC	3
466			min	-.001	3	-.419	1	.006	15	-9.238e-3	1	476.698	1	1048.226	1
467		6	max	.006	1	.351	3	.163	1	8.146e-3	3	NC	5	NC	3
468			min	-.001	3	-.388	1	.006	15	-1.03e-2	1	516.928	1	1104.167	1
469		7	max	.006	1	.281	3	.124	1	8.974e-3	3	NC	5	NC	3
470			min	-.001	3	-.314	1	.002	10	-1.136e-2	1	651.957	1	1446.737	1
471		8	max	.006	1	.191	3	.064	1	9.803e-3	3	NC	5	NC	2
472			min	-.001	3	-.216	1	-.004	10	-1.242e-2	1	991.36	1	2683.504	1
473		9	max	.005	1	.107	3	.017	3	1.063e-2	3	NC	4	NC	1
474			min	-.001	3	-.126	1	-.013	2	-1.348e-2	1	1901.743	1	NC	1
475		10	max	.005	1	.069	3	.018	3	1.146e-2	3	NC	4	NC	1
476			min	-.001	3	-.085	1	-.024	2	-1.455e-2	1	3271.358	1	NC	1
477		11	max	.005	1	.107	3	.021	3	1.063e-2	3	NC	4	NC	1
478			min	-.001	3	-.126	1	-.012	2	-1.348e-2	1	1901.744	1	NC	1
479		12	max	.005	1	.191	3	.07	1	9.804e-3	3	NC	5	NC	3
480			min	-.001	3	-.216	1	-.004	10	-1.242e-2	1	991.36	1	2476.965	1
481		13	max	.005	1	.281	3	.13	1	8.976e-3	3	NC	5	NC	5
482			min	-.001	3	-.314	1	.002	10	-1.136e-2	1	651.957	1	1373.333	1
483		14	max	.005	1	.351	3	.17	1	8.147e-3	3	NC	5	NC	5
484			min	-.001	3	-.388	1	.006	15	-1.03e-2	1	516.928	1	1060.029	1
485		15	max	.005	1	.38	3	.179	1	7.319e-3	3	NC	5	NC	3
486			min	-.001	3	-.419	1	.006	15	-9.237e-3	1	476.699	1	1011.773	1
487		16	max	.005	1	.358	3	.154	1	6.491e-3	3	NC	5	NC	3
488			min	-.001	3	-.394	1	.005	15	-8.175e-3	1	509.05	1	1173.371	1
489		17	max	.005	1	.284	3	.102	1	5.663e-3	3	NC	5	NC	3
490			min	-.001	3	-.313	1	.004	15	-7.114e-3	1	653.952	1	1741.441	1
491		18	max	.005	1	.167	3	.04	1	4.834e-3	3	NC	5	NC	2
492			min	-.001	3	-.185	1	0	10	-6.052e-3	1	1190.054	1	4103.852	1
493		19	max	.005	1	.023	3	.006	3	4.006e-3	3	NC	1	NC	1
494			min	-.001	3	-.028	1	-.007	2	-4.99e-3	1	NC	1	NC	1
495	M16	1	max	.002	1	.018	3	.006	3	5.234e-3	1	NC	1	NC	1
496			min	0	3	-.031	1	-.008	2	-3.048e-3	3	NC	1	NC	1
497		2	max	.002	1	.085	3	.041	1	6.382e-3	1	NC	5	NC	2
498			min	0	3	-.208	1	0	10	-3.648e-3	3	1048.492	1	4019.433	1
499		3	max	.002	1	.14	3	.102	1	7.529e-3	1	NC	5	NC	3
500			min	0	3	-.354	1	.004	15	-4.248e-3	3	576.211	1	1723.564	1
501		4	max	.002	1	.175	3	.154	1	8.677e-3	1	NC	5	NC	3
502			min	0	3	-.446	1	.005	15	-4.849e-3	3	448.605	1	1168.203	1
503		5	max	.002	1	.186	3	.178	1	9.824e-3	1	NC	5	NC	3
504			min	0	3	-.474	1	.006	15	-5.449e-3	3	420.207	1	1011.96	1
505		6	max	.002	1	.175	3	.169	1	1.097e-2	1	NC	5	NC	5
506			min	0	3	-.439	1	.006	15	-6.049e-3	3	455.877	1	1065.278	1
507		7	max	.002	1	.145	3	.128	1	1.212e-2	1	NC	5	NC	3



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
508		min	0	3	-354	1	.002	10	-6.649e-3	3	575.455	1	1389.304	1
509	8	max	.003	1	.105	3	.068	1	1.327e-2	1	NC	5	NC	2
510		min	0	3	-.243	1	-.004	10	-7.249e-3	3	876.681	1	2540.478	1
511	9	max	.003	1	.068	3	.02	3	1.441e-2	1	NC	4	NC	1
512		min	0	3	-.141	1	-.013	2	-7.849e-3	3	1689.926	1	NC	1
513	10	max	.003	1	.051	3	.018	3	1.556e-2	1	NC	4	NC	1
514		min	0	3	-.094	1	-.024	2	-8.449e-3	3	2928.143	1	NC	1
515	11	max	.003	1	.068	3	.018	3	1.441e-2	1	NC	4	NC	1
516		min	0	3	-.141	1	-.013	2	-7.849e-3	3	1689.926	1	NC	1
517	12	max	.003	1	.105	3	.066	1	1.327e-2	1	NC	5	NC	3
518		min	0	3	-.243	1	-.004	10	-7.248e-3	3	876.681	1	2612.045	1
519	13	max	.003	1	.145	3	.126	1	1.212e-2	1	NC	5	NC	3
520		min	0	3	-354	1	.002	10	-6.648e-3	3	575.455	1	1419.404	1
521	14	max	.003	1	.175	3	.166	1	1.097e-2	1	NC	5	NC	3
522		min	0	3	-.439	1	.006	15	-6.047e-3	3	455.877	1	1086.709	1
523	15	max	.003	1	.186	3	.175	1	9.826e-3	1	NC	5	NC	3
524		min	0	3	-.474	1	.006	15	-5.447e-3	3	420.207	1	1032.935	1
525	16	max	.003	1	.175	3	.15	1	8.678e-3	1	NC	5	NC	3
526		min	0	3	-.446	1	.005	15	-4.846e-3	3	448.605	1	1195.372	1
527	17	max	.003	1	.14	3	.099	1	7.531e-3	1	NC	5	NC	3
528		min	0	3	-354	1	.003	15	-4.245e-3	3	576.212	1	1773.133	1
529	18	max	.003	1	.085	3	.039	1	6.384e-3	1	NC	5	NC	2
530		min	0	3	-.208	1	0	10	-3.645e-3	3	1048.493	1	4185.822	1
531	19	max	.003	1	.018	3	.006	3	5.237e-3	1	NC	1	NC	1
532		min	0	3	-.031	1	-.008	2	-3.044e-3	3	NC	1	NC	1
533	M15	1	max	0	0	1	0	1	3.215e-4	3	NC	1	NC	1
534		min	0	1	0	1	0	1	-8.855e-5	2	NC	1	NC	1
535	2	max	0	3	-.005	15	.001	1	8.178e-4	3	NC	5	NC	1
536		min	0	10	-.021	4	0	3	-7.461e-4	1	4688.414	4	NC	1
537	3	max	0	3	-.01	15	.004	1	1.314e-3	3	NC	15	NC	1
538		min	0	10	-.042	4	-.003	3	-1.42e-3	1	2385.774	4	NC	1
539	4	max	0	3	-.014	15	.008	1	1.81e-3	3	6963.098	15	NC	3
540		min	0	10	-.061	4	-.006	3	-2.093e-3	1	1636.78	4	8345.246	1
541	5	max	0	3	-.018	15	.013	1	2.307e-3	3	5433.376	15	NC	4
542		min	0	10	-.078	4	-.01	3	-2.767e-3	1	1277.196	4	5471.546	1
543	6	max	0	3	-.022	15	.019	1	2.803e-3	3	4572.76	15	NC	4
544		min	0	10	-.092	4	-.015	3	-3.44e-3	1	1074.896	4	3981.098	1
545	7	max	0	3	-.024	15	.024	1	3.299e-3	3	4055.213	15	NC	4
546		min	0	10	-.104	4	-.019	3	-4.114e-3	1	953.238	4	3110.479	1
547	8	max	0	3	-.026	15	.03	1	3.796e-3	3	3744.607	15	NC	4
548		min	0	10	-.113	4	-.024	3	-4.787e-3	1	880.226	4	2563.579	1
549	9	max	0	3	-.028	15	.035	1	4.292e-3	3	3577.421	15	NC	4
550		min	0	10	-.118	4	-.028	3	-5.461e-3	1	840.926	4	2205.851	1
551	10	max	0	3	-.028	15	.039	1	4.788e-3	3	3524.531	15	NC	4
552		min	0	10	-.12	4	-.031	3	-6.134e-3	1	828.494	4	1969.785	1
553	11	max	0	3	-.028	15	.042	1	5.284e-3	3	3577.421	15	NC	5
554		min	0	10	-.118	4	-.034	3	-6.808e-3	1	840.926	4	1819.955	1
555	12	max	0	3	-.026	15	.043	1	5.781e-3	3	3744.607	15	NC	5
556		min	0	10	-.113	4	-.035	3	-7.481e-3	1	880.226	4	1738.77	1
557	13	max	0	3	-.024	15	.042	1	6.277e-3	3	4055.213	15	NC	5
558		min	0	10	-.105	4	-.034	3	-8.155e-3	1	953.238	4	1721.082	1
559	14	max	0	3	-.022	15	.039	1	6.773e-3	3	4572.76	15	NC	5
560		min	0	10	-.093	4	-.032	3	-8.828e-3	1	1074.896	4	1774.335	1
561	15	max	0	3	-.018	15	.034	1	7.27e-3	3	5433.376	15	NC	4
562		min	0	10	-.078	4	-.027	3	-9.502e-3	1	1277.196	4	1926.043	1
563	16	max	0	3	-.014	15	.025	1	7.766e-3	3	6963.098	15	NC	4
564		min	0	10	-.062	4	-.019	3	-1.018e-2	1	1636.78	4	2250.992	1



Company : Schletter, Inc.
Designer : HCV
Job Number :
Model Name : Standard PVMini Racking System

Dec 11, 2015

Checked By: _____

Envelope Member Section Deflections (Continued)

Member	Sec		x [in]	LC	y [in]	LC	z [in]	LC	x Rotate [r...	LC	(n) L/y Ratio	LC	(n) L/z Ratio	LC
565	17	max	0	3	-.01	15	.013	1	8.262e-3	3	NC	15	NC	4
566		min	0	10	-.043	4	-.009	3	-1.085e-2	1	2385.774	4	2983.872	1
567	18	max	0	3	-.005	15	.004	3	8.759e-3	3	NC	5	NC	4
568		min	0	10	-.022	4	-.008	2	-1.152e-2	1	4688.414	4	5311.981	1
569	19	max	0	3	.004	3	.021	3	9.255e-3	3	NC	1	NC	1
570		min	0	10	-.004	1	-.025	2	-1.22e-2	1	NC	1	NC	1
571	M16A	1	max	0	0	3	.007	3	3.14e-3	3	NC	1	NC	1
572		min	0	3	-.002	1	-.008	2	-3.76e-3	1	NC	1	NC	1
573	2	max	0	10	-.005	15	.005	1	3.001e-3	3	NC	5	NC	2
574		min	0	3	-.022	4	-.001	10	-3.577e-3	1	4688.414	4	9202.613	1
575	3	max	0	10	-.01	15	.014	1	2.861e-3	3	NC	15	NC	4
576		min	0	3	-.042	4	-.004	3	-3.394e-3	1	2385.774	4	5201.746	1
577	4	max	0	10	-.014	15	.02	1	2.721e-3	3	6963.098	15	NC	4
578		min	0	3	-.061	4	-.008	3	-3.211e-3	1	1636.78	4	3951.916	1
579	5	max	0	10	-.018	15	.024	1	2.582e-3	3	5433.376	15	NC	4
580		min	0	3	-.078	4	-.01	3	-3.028e-3	1	1277.196	4	3408.695	1
581	6	max	0	10	-.022	15	.027	1	2.442e-3	3	4572.76	15	NC	4
582		min	0	3	-.092	4	-.012	3	-2.845e-3	1	1074.896	4	3169.305	1
583	7	max	0	10	-.024	15	.028	1	2.302e-3	3	4055.213	15	NC	4
584		min	0	3	-.104	4	-.013	3	-2.662e-3	1	953.238	4	3107.267	1
585	8	max	0	10	-.026	15	.027	1	2.163e-3	3	3744.607	15	NC	4
586		min	0	3	-.113	4	-.013	3	-2.479e-3	1	880.226	4	3178.915	1
587	9	max	0	10	-.028	15	.026	1	2.023e-3	3	3577.421	15	NC	4
588		min	0	3	-.118	4	-.012	3	-2.296e-3	1	840.926	4	3377.545	1
589	10	max	0	10	-.028	15	.023	1	1.883e-3	3	3524.531	15	NC	4
590		min	0	3	-.12	4	-.011	3	-2.113e-3	1	828.494	4	3722.541	1
591	11	max	0	10	-.028	15	.02	1	1.744e-3	3	3577.421	15	NC	4
592		min	0	3	-.118	4	-.009	3	-1.93e-3	1	840.926	4	4263.275	1
593	12	max	0	10	-.026	15	.017	1	1.604e-3	3	3744.607	15	NC	4
594		min	0	3	-.112	4	-.008	3	-1.747e-3	1	880.226	4	5097.804	1
595	13	max	0	10	-.024	15	.013	1	1.464e-3	3	4055.213	15	NC	3
596		min	0	3	-.104	4	-.006	3	-1.564e-3	1	953.238	4	6420.759	1
597	14	max	0	10	-.022	15	.01	1	1.325e-3	3	4572.76	15	NC	2
598		min	0	3	-.092	4	-.004	3	-1.381e-3	1	1074.896	4	8649.48	1
599	15	max	0	10	-.018	15	.006	1	1.185e-3	3	5433.376	15	NC	1
600		min	0	3	-.077	4	-.003	3	-1.198e-3	1	1277.196	4	NC	1
601	16	max	0	10	-.014	15	.003	1	1.045e-3	3	6963.098	15	NC	1
602		min	0	3	-.06	4	-.001	3	-1.016e-3	2	1636.78	4	NC	1
603	17	max	0	10	-.01	15	.001	1	9.055e-4	3	NC	15	NC	1
604		min	0	3	-.041	4	0	3	-8.578e-4	2	2385.774	4	NC	1
605	18	max	0	10	-.005	15	0	4	7.658e-4	3	NC	5	NC	1
606		min	0	3	-.021	4	0	2	-6.999e-4	2	4688.414	4	NC	1
607	19	max	0	1	0	1	0	1	6.261e-4	3	NC	1	NC	1
608		min	0	1	0	1	0	1	-5.421e-4	2	NC	1	NC	1



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 4.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N_{ua} (lb)	Shear load x, V_{uax} (lb)	Shear load y, V_{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	405.0	6.0	101.0	101.2
Sum	405.0	6.0	101.0	101.2

Maximum concrete compression strain (%): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 405
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N_{sa} (lb)	ϕ	ϕN_{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k_c	λ	f_c (psi)	h_{ef} (in)	N_b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cb} = \phi (A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 & Eq. D-4)}$$

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	ϕ	ϕN_{cb} (lb)
253.92	256.00	0.995	1.00	1.000	10469	0.65	6717

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

$\tau_{k,cr}$ (psi)	$f_{short-term}$	K_{sat}	$\tau_{k,cr}$ (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

$\tau_{k,cr}$ (psi)	d_a (in)	h_{ef} (in)	N_{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_a = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 & Eq. D-16a)}$$

A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	ϕ	ϕN_a (lb)
109.66	109.66	1.000	1.000	9755	0.55	5365

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in y-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	0.897	1.000	1.000	8488	0.70	4411

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	0.903	1.000	1.000	8282	0.70	3549

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{by} (lb)	ϕ	ϕV_{cbx} (lb)
238.44	288.00	1.000	1.000	1.000	8488	0.70	9838

Shear parallel to edge in y-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{a1}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{a1} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	7.87	8282

$$\phi V_{cbx} = \phi (2)(A_{Vc} / A_{Vco}) \psi_{ed,V} \psi_{c,V} \psi_{h,V} V_{bx} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-21)}$$

A_{Vc} (in ²)	A_{Vco} (in ²)	$\psi_{ed,V}$	$\psi_{c,V}$	$\psi_{h,V}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
188.88	278.72	1.000	1.000	1.000	8282	0.70	7858

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cp} = \phi \min[k_{cp} N_a; k_{cp} N_{cb}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30a)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	109.66	109.66	1.000	1.000	9755	9755

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ	ϕV_{cp} (lb)
253.92	256.00	0.995	1.000	1.000	10469	10334	0.70	13657

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N_{ua} (lb)	Design Strength, ϕN_n (lb)	Ratio	Status	
Steel	405	6071	0.07	Pass	
Concrete breakout	405	6717	0.06	Pass	
Adhesive	405	5365	0.08	Pass (Governs)	
Shear	Factored Load, V_{ua} (lb)	Design Strength, ϕV_n (lb)	Ratio	Status	
Steel	101	3156	0.03	Pass (Governs)	
T Concrete breakout y+	101	4411	0.02	Pass	
T Concrete breakout x+	6	3549	0.00	Pass	
Concrete breakout y+	6	9838	0.00	Pass	
Concrete breakout x+	101	7858	0.01	Pass	
Concrete breakout, combined	-	-	0.02	Pass	
Pryout	101	13657	0.01	Pass	
Interaction check	$N_{ua}/\phi N_n$	$V_{ua}/\phi V_n$	Combined Ratio	Permissible	Status
Sec. D.7.1	0.08	0.00	7.5 %	1.0	Pass

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	1/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

1. Project information

Customer company:
Customer contact name:
Customer e-mail:
Comment:

Project description:
Location:
Fastening description:

2. Input Data & Anchor Parameters

General

Design method: ACI 318-05
Units: Imperial units

Anchor Information:

Anchor type: Bonded anchor
Material: A193 Grade B8/B8M (304/316SS)
Diameter (inch): 0.500
Effective Embedment depth, h_{ef} (inch): 6.000
Code report: IAPMO UES ER-263
Anchor category: -
Anchor ductility: Yes
 h_{min} (inch): 8.50
 C_{ac} (inch): 9.67
 C_{min} (inch): 1.75
 S_{min} (inch): 3.00

Load and Geometry

Load factor source: ACI 318 Section 9.2
Load combination: not set
Seismic design: No
Anchors subjected to sustained tension: No
Apply entire shear load at front row: No
Anchors only resisting wind and/or seismic loads: No

Base Material

Concrete: Normal-weight
Concrete thickness, h (inch): 18.00
State: Cracked
Compressive strength, f'_c (psi): 2500
 $\Psi_{c,v}$: 1.0
Reinforcement condition: B tension, B shear
Supplemental reinforcement: Not applicable
Reinforcement provided at corners: No
Do not evaluate concrete breakout in tension: No
Do not evaluate concrete breakout in shear: No
Hole condition: Dry concrete
Inspection: Periodic
Temperature range, Short/Long: 110/75°F
Ignore 6do requirement: Not applicable
Build-up grout pad: No

Base Plate

Length x Width x Thickness (inch): 9.00 x 4.00 x 0.28

<Figure 1>



Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	2/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

<Figure 2>



Recommended Anchor

Anchor Name: AT-XP® - AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS)
Code Report: IAPMO UES ER-263





Anchor Designer™ Software Version 2.4.5673.0

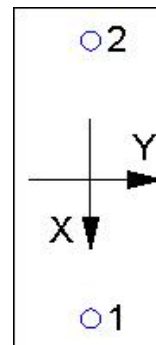
Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	3/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

3. Resulting Anchor Forces

Anchor	Tension load, N _{ua} (lb)	Shear load x, V _{uax} (lb)	Shear load y, V _{uay} (lb)	Shear load combined, $\sqrt{(V_{uax})^2 + (V_{uay})^2}$ (lb)
1	732.5	499.5	0.0	499.5
2	732.5	499.5	0.0	499.5
Sum	1465.0	999.0	0.0	999.0

Maximum concrete compression strain (‰): 0.00
Maximum concrete compression stress (psi): 0
Resultant tension force (lb): 1465
Resultant compression force (lb): 0
Eccentricity of resultant tension forces in x-axis, e'_{Nx} (inch): 0.00
Eccentricity of resultant tension forces in y-axis, e'_{Ny} (inch): 0.00
Eccentricity of resultant shear forces in x-axis, e'_{Vx} (inch): 0.00
Eccentricity of resultant shear forces in y-axis, e'_{Vy} (inch): 0.00

<Figure 3>



4. Steel Strength of Anchor in Tension (Sec. D.5.1)

N _{sa} (lb)	φ	φN _{sa} (lb)
8095	0.75	6071

5. Concrete Breakout Strength of Anchor in Tension (Sec. D.5.2)

$$N_b = k_c \lambda \sqrt{f'_c} h_{ef}^{1.5} \text{ (Eq. D-7)}$$

k _c	λ	f' _c (psi)	h _{ef} (in)	N _b (lb)
17.0	1.00	2500	5.333	10469

$$\phi N_{cbg} = \phi (A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b \text{ (Sec. D.4.1 \& Eq. D-5)}$$

A _{Nc} (in ²)	A _{Nco} (in ²)	ψ _{ec,N}	ψ _{ed,N}	ψ _{c,N}	ψ _{cp,N}	N _b (lb)	φ	φN _{cbg} (lb)
314.72	256.00	1.000	0.865	1.00	1.000	10469	0.65	7233

6. Adhesive Strength of Anchor in Tension (AC308 Sec. 3.3)

$$\tau_{k,cr} = \tau_{k,cr} f_{short-term} K_{sat}$$

τ _{k,cr} (psi)	f _{short-term}	K _{sat}	τ _{k,cr} (psi)
1035	1.00	1.00	1035

$$N_{a0} = \tau_{k,cr} \pi d_a h_{ef} \text{ (Eq. D-16f)}$$

τ _{k,cr} (psi)	d _a (in)	h _{ef} (in)	N _{a0} (lb)
1035	0.50	6.000	9755

$$\phi N_{ag} = \phi (A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0} \text{ (Sec. D.4.1 \& Eq. D-16b)}$$

A _{Na} (in ²)	A _{Na0} (in ²)	ψ _{ed,Na}	ψ _{g,Na}	ψ _{ec,Na}	ψ _{p,Na}	N _{a0} (lb)	φ	φN _{ag} (lb)
177.03	109.66	0.952	1.021	1.000	1.000	9755	0.55	8418

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	4/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

8. Steel Strength of Anchor in Shear (Sec. D.6.1)

V_{sa} (lb)	ϕ_{grout}	ϕ	$\phi_{grout}\phi V_{sa}$ (lb)
4855	1.0	0.65	3156

9. Concrete Breakout Strength of Anchor in Shear (Sec. D.6.2)

Shear perpendicular to edge in x-direction:

$$V_{bx} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{bx} (lb)
4.00	0.50	1.00	2500	12.00	15593

$$\phi V_{cbx} = \phi (A_{vc} / A_{vco}) \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{bx} \text{ (Sec. D.4.1 & Eq. D-21)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{bx} (lb)	ϕ	ϕV_{cbx} (lb)
288.00	648.00	0.833	1.000	1.000	15593	0.70	4043

Shear parallel to edge in x-direction:

$$V_{by} = 7(l_e / d_a)^{0.2} \sqrt{d_a \lambda} \sqrt{f'_c c_{at}}^{1.5} \text{ (Eq. D-24)}$$

l_e (in)	d_a (in)	λ	f'_c (psi)	c_{at} (in)	V_{by} (lb)
4.00	0.50	1.00	2500	8.00	8488

$$\phi V_{cbgx} = \phi (2)(A_{vc} / A_{vco}) \psi_{ec,v} \psi_{ed,v} \psi_{c,v} \psi_{h,v} V_{by} \text{ (Sec. D.4.1, D.6.2.1(c) & Eq. D-22)}$$

A_{vc} (in ²)	A_{vco} (in ²)	$\psi_{ec,v}$	$\psi_{ed,v}$	$\psi_{c,v}$	$\psi_{h,v}$	V_{by} (lb)	ϕ	ϕV_{cbgx} (lb)
284.04	288.00	1.000	1.000	1.000	1.000	8488	0.70	11720

10. Concrete Pryout Strength of Anchor in Shear (Sec. D.6.3)

$$\phi V_{cpq} = \phi \min[k_{cp} N_{ag}; k_{cp} N_{cbg}] = \phi \min[k_{cp}(A_{Na} / A_{Na0}) \psi_{ed,Na} \psi_{g,Na} \psi_{ec,Na} \psi_{p,Na} N_{a0}; k_{cp}(A_{Nc} / A_{Nco}) \psi_{ec,N} \psi_{ed,N} \psi_{c,N} \psi_{cp,N} N_b] \text{ (Eq. D-30b)}$$

k_{cp}	A_{Na} (in ²)	A_{Na0} (in ²)	$\psi_{ed,Na}$	$\psi_{g,Na}$	$\psi_{ec,Na}$	$\psi_{p,Na}$	N_{a0} (lb)	N_a (lb)
2.0	177.03	109.66	0.952	1.021	1.000	1.000	9755	15305

A_{Nc} (in ²)	A_{Nco} (in ²)	$\psi_{ec,N}$	$\psi_{ed,N}$	$\psi_{c,N}$	$\psi_{cp,N}$	N_b (lb)	N_{cb} (lb)	ϕ
314.72	256.00	1.000	0.865	1.000	1.000	10469	11128	0.70

ϕV_{cpq} (lb)
15580

11. Results

Interaction of Tensile and Shear Forces (Sec. D.7)

Tension	Factored Load, N _{ua} (lb)	Design Strength, ϕN _n (lb)	Ratio	Status	
Steel	733	6071	0.12	Pass	
Concrete breakout	1465	7233	0.20	Pass (Governs)	
Adhesive	1465	8418	0.17	Pass	
Shear	Factored Load, V _{ua} (lb)	Design Strength, ϕV _n (lb)	Ratio	Status	
Steel	500	3156	0.16	Pass	
T Concrete breakout x+	999	4043	0.25	Pass (Governs)	
Concrete breakout y-	999	11720	0.09	Pass (Governs)	
Pryout	999	15580	0.06	Pass	
Interaction check	N _{ua} /ϕN _n	V _{ua} /ϕV _n	Combined Ratio	Permissible	Status

Input data and results must be checked for agreement with the existing circumstances, the standards and guidelines must be checked for plausibility.

Simpson Strong-Tie Company Inc. 5956 W. Las Positas Boulevard Pleasanton, CA 94588 Phone: 925.560.9000 Fax: 925.847.3871 www.strongtie.com



Anchor Designer™
Software
Version 2.4.5673.0

Company:	Schletter, Inc.	Date:	12/10/2015
Engineer:	HCV	Page:	5/5
Project:	Standard PVMini - Worst Case		
Address:			
Phone:			
E-mail:			

Sec. D.7.3	0.20	0.25	45.0 %	1.2	Pass
------------	------	------	--------	-----	------

AT-XP w/ 1/2"Ø A193 Gr. B8/B8M (304/316SS) with hef = 6.000 inch meets the selected design criteria.

12. Warnings

- This temperature range is currently outside the scope of ACI 318-11 and ACI 355.4, and is provided for historical purposes.
- Designer must exercise own judgement to determine if this design is suitable.
- Refer to manufacturer's product literature for hole cleaning and installation instructions.